

APPENDIX I

SW 10th Street Connector & I-95 Interchange Supplemental Traffic Forecast Scenarios

SW 10th Street Eastbound Weave Operations from the Connector Lane Egress, West of Newport Center Drive, to the Newport Center Drive and I-95 Intersections

SW 10th Street at I-95 – Alternatives Analysis Memorandum



MEMORANDUM

Date: July 20, 2020

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Subject: **SW 10th Street Connector & I-95 Interchange Supplemental Traffic Forecast Scenarios**

FPN(s): 436964-1, 439891-1

Counties: Broward (86)

At the request of the Florida Department of Transportation (FDOT) District 4, Florida's Turnpike Enterprise (FTE) staff was tasked with evaluating additional forecast scenarios addressing potential modifications to the SW 10th Street Connector ramps to and from I-95. With the changes in traffic demand associated with the I-95 connections, the intermediate access ramp connections between the Florida's Turnpike and I-95 interchanges were also revisited and new forecast scenarios were developed. Furthermore, the Sawgrass Expressway/Turnpike interchange configuration has been revised since the previous forecast, resulting in a modified forecast affecting the new scenarios.

This memorandum is prepared in support of the I-95 Project Development and Environment (PD&E) study from SW 10th Street to Hillsboro Boulevard (FPID 436964-1) and the SW 10th Street Connector PD&E study (FPID 439891-1). This memorandum provides supplemental traffic forecast scenarios to the *Project Traffic Forecast Memorandum (PTFM)*, dated January 2019, and the *SW 10th Street Connector Toll-Free Project Traffic Forecast Memorandum*, dated November 2019. The Alternative 3D 1.3b non-tolled forecast from the November 2019 memo is now referred to as "Base PD&E Concept" since this forecast was used in the draft *SW 10th Street Connector PD&E Project Traffic Analysis Report (PTAR)*, dated September 2019.

Since the publication of the draft *SW 10th Street Connector PD&E PTAR*, FTE has revised the interchange configuration at the Sawgrass Expressway/Turnpike interchange to remove the express lanes along the Sawgrass Expressway, and change the Turnpike Mainline configuration from two express lanes in each direction to one managed lane in each direction. The direct

connect ramps to/from the Turnpike south and from/to the SW 10th Street Connector east were changed to connect to the Turnpike general lanes only, instead of the Turnpike express lanes only. These changes are reflected in each new forecast scenario for the SW 10th Street Connector.

For simplification purposes, new scenario names were established in this memo rather than retaining previously named forecast scenario names. Scenarios A, B, and C describe three basic intermediate access options for the SW 10th Street Connector, as described below:

Scenario A: Provides intermediate access for local SW 10th Street, serving Powerline Road (to/from the east) and Newport Center Drive (to/from the west).

Scenario B: Provides no access between local SW 10th Street and the SW 10th Street Connector; no ramps to serve either Powerline Road or Newport Center Drive.

Scenario C: Provides access east of Military Trail for local SW 10th Street, serving Newport Center Drive to/from the west. There is no access to/from Powerline Road.

For each scenario, three sub-scenarios describe the connection possibilities for the SW 10th Street Connector with the I-95 general use lanes (GULs) and express lanes (ELs), as describe below:

Sub-scenario 1: Connects to/from I-95 ELs only.

Sub-scenario 2: Connects to/from I-95 GULs, as well as I-95 ELs.

Sub-scenario 3: Connects to/from I-95 GULs only.

The scenario and sub-scenarios combinations create nine unique forecast scenarios (A1, A2, A3, B1, B2, B3, C1, C2, and C3), as summarized in **Table 1. Appendix A** also provides a schematic for each scenario east of the Sawgrass Expressway/Turnpike interchange. **Appendix B** provides the consolidated diagram with Sawgrass Expressway, Turnpike, SW 10th Street, and I-95 for the previous Base PD&E Concept and Scenario A2. The latest update of the Turnpike/Sawgrass interchange as shown in A2 can be described as follows:

- Sawgrass Expressway: 5 GTLs instead of 3 GTLs + 2 ELs
- Turnpike Mainline: 1 Managed Lane + 4 GTLs instead of 2 ELs + 3 GTLs
- Sawgrass/Turnpike Interchange: No EL Direct Connect
- Turnpike to SW 10th Connector (from south to east): GTLs connection instead of ELs

Initial Directional Design Hour Volumes (DDHVs) were developed for the 2040 AM period only to compare and shortlist these scenarios for further analysis. This approach is approved by the study team.

Table 1: SW 10th Street Connector Forecast Scenarios Summary

Scenario	Turnpike Interchange		Intermediate Access between Turnpike and I-95		I-95 Interchange Connection		
	GUL	EL	East of Powerline Rd.	East of Military Tr.	EL	EL & GUL	GUL
PD&E Base	X	X	X	X	X		
A1	X		X	X	X		
A2	X		X	X		X	
A3	X		X	X			X
B1	X				X		
B2	X					X	
B3	X						X
C1	X			X	X		
C2	X			X		X	
C3	X			X			X

The traffic forecasting process was accomplished using the Express Lane Time-of-Day (ELToD) model to identify traffic volume split between connector lanes and local lanes. The ELToD model encompasses the area of three study corridors:

- Florida’s Turnpike corridor between Lake Worth Road and Atlantic Boulevard
- I-95 corridor between Congress Avenue and Atlantic Boulevard
- Sawgrass/SW 10th Street corridor between University Drive and Natura Boulevard

The trip matrices from the Southeast Regional Planning Model (SERPM) were used as input to the subarea ELToD Model. Trip matrices for Scenarios A2/C2 are provided in **Appendix C**. Each of the scenarios was coded into ELToD to estimate the managed lane and local lane traffic. ELToD can model toll and non-toll portions of the system in one cohesive model network. In these forecast scenarios, the only express lane portion is I-95 Express since the other priced managed lane toll components were removed from SW 10th Street, Sawgrass Expressway, and the Turnpike Mainline.

Figure 1 through 3 present the SW 10th Street Connector and local lane 2040 AM DDHVs along SW 10th Street between the Turnpike and I-95 for each scenario in comparison to the Base PD&E Concept. The SW 10th Street local lane volumes for both directions are shown at three locations: west of Powerline Road, west of Military Trail, and west of Newport Center Drive.

Figure 1: Year 2040 AM Peak Hour Scenario A Traffic Forecasts

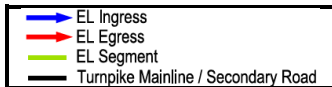
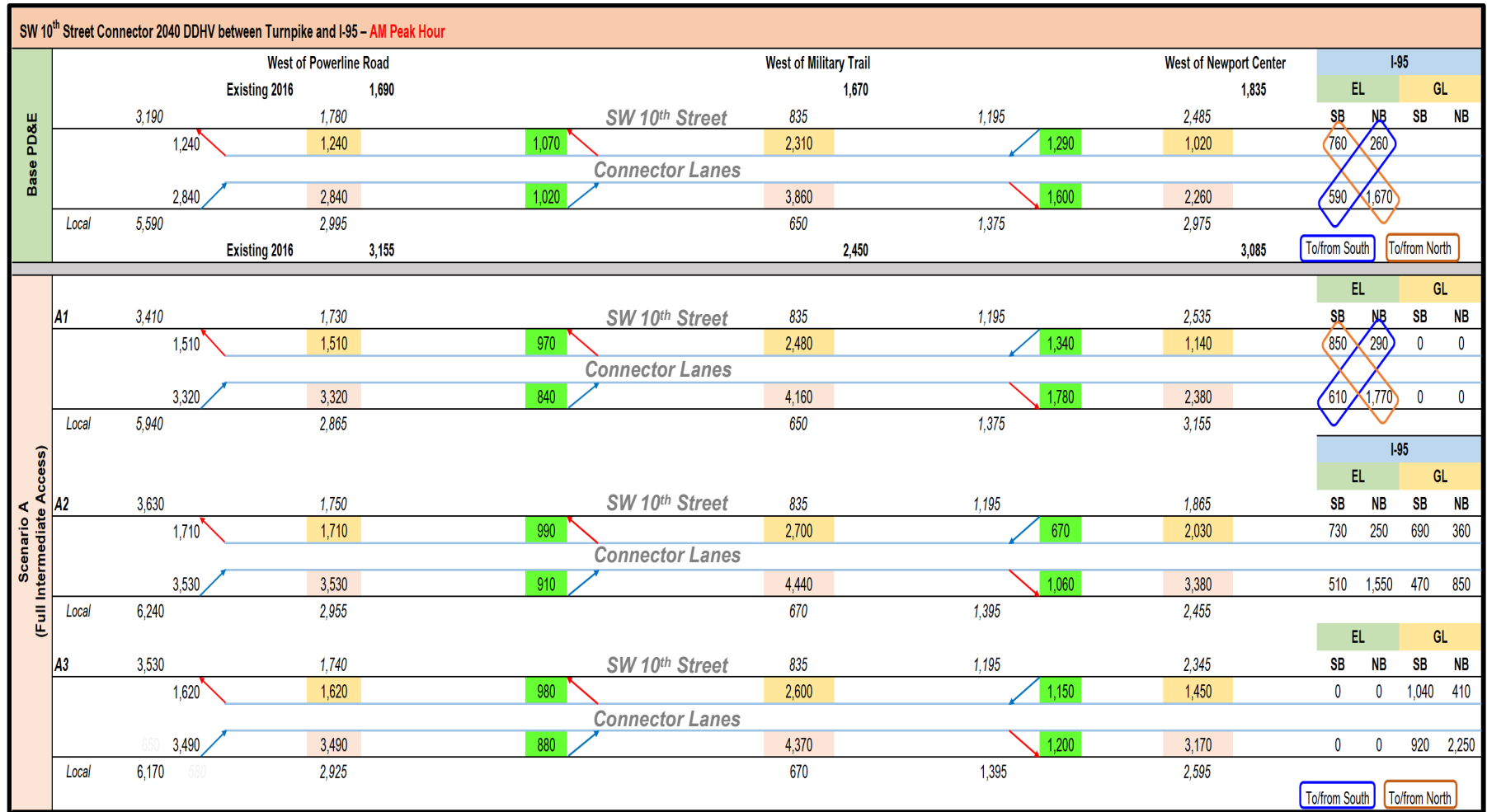


Figure 2: Year 2040 AM Peak Hour Scenario B Traffic Forecasts

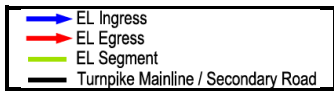
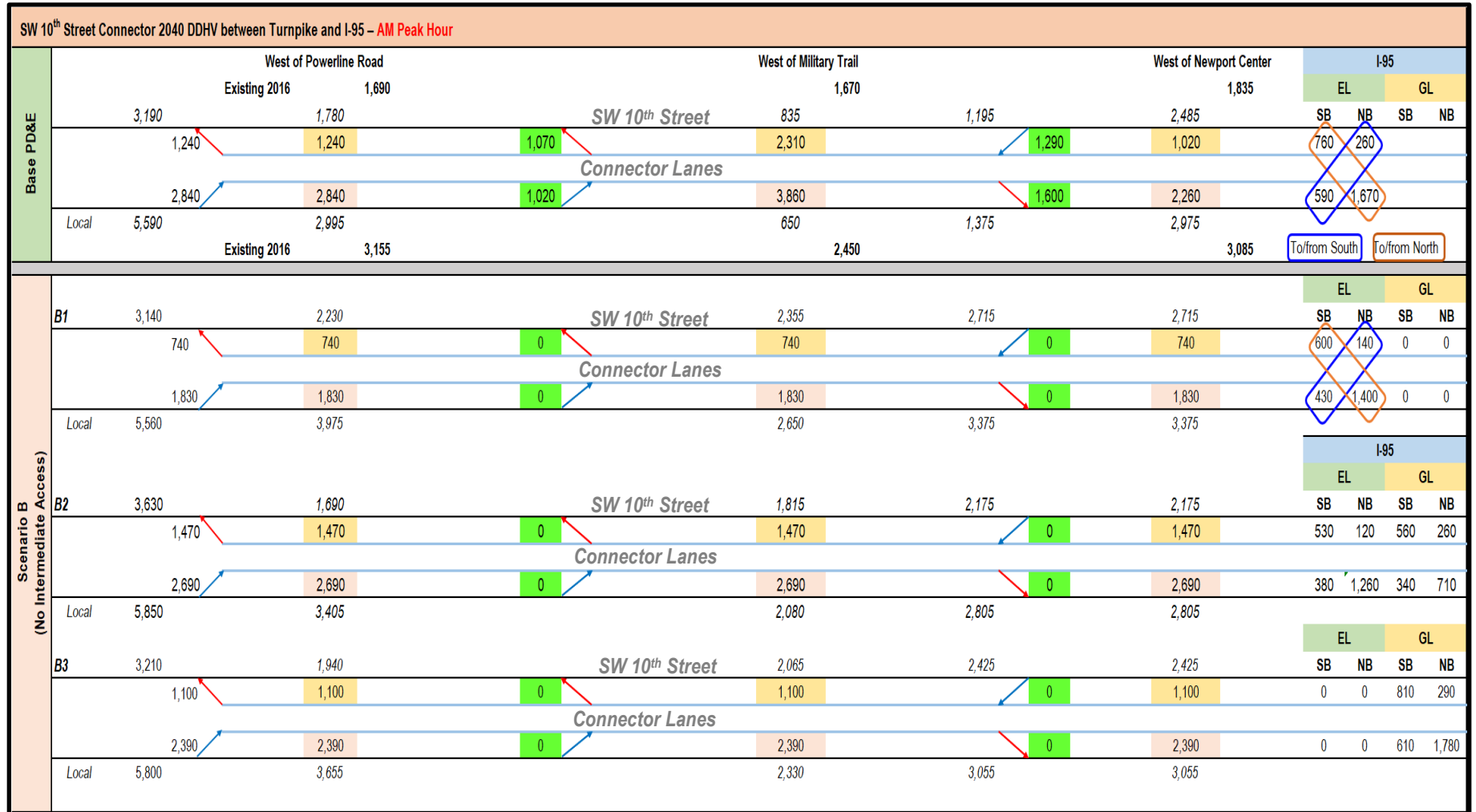
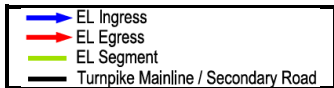
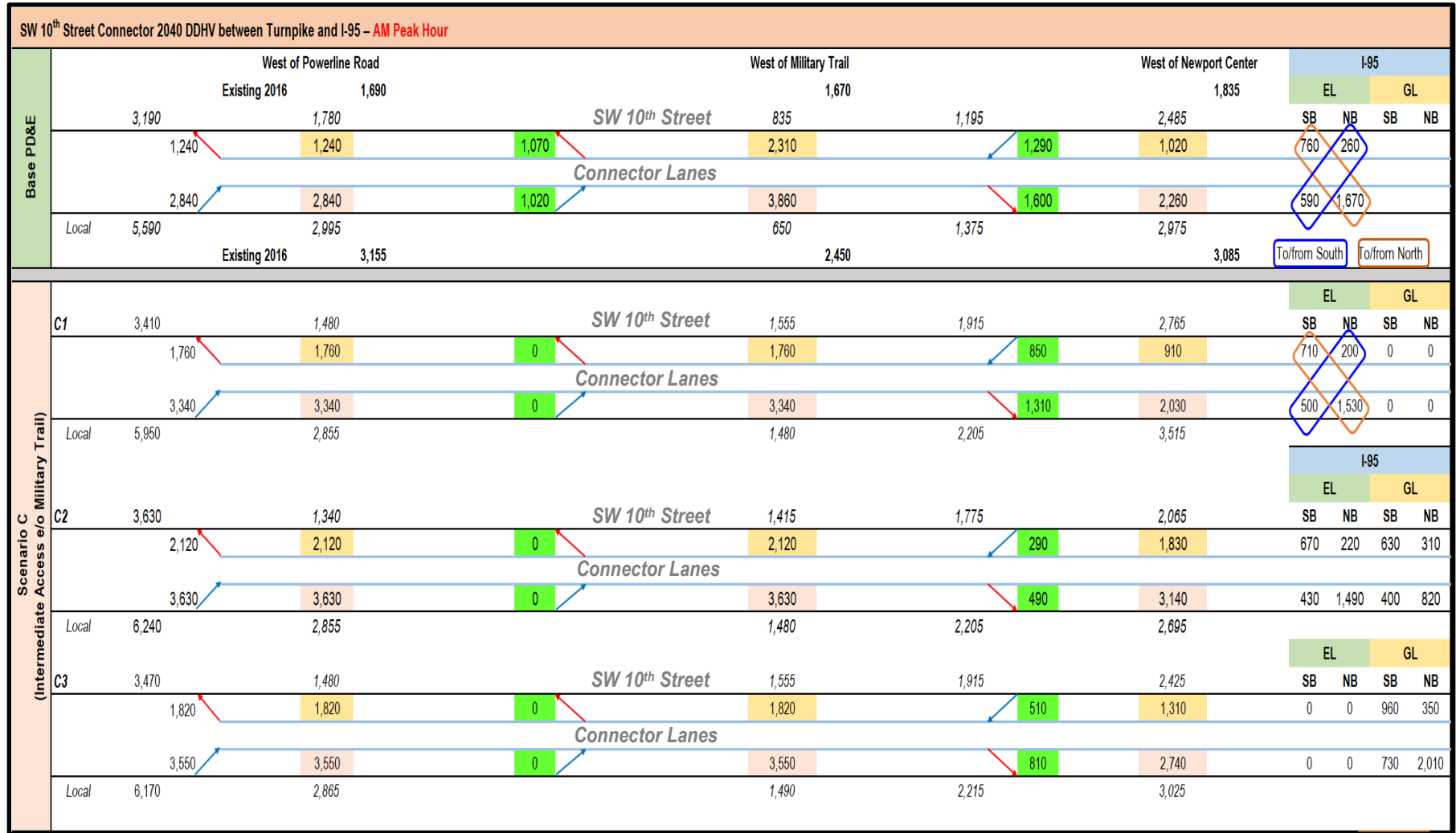


Figure 3: Year 2040 AM Peak Hour Scenario C Traffic Forecasts



An important aspect of the new scenarios is the connection to the I-95 GULs. With the general use connection, the anticipated volume on the SW 10th Street Connector increases, and local lane traffic decreases. Additionally, the connection to the I-95 GULs also affects the traffic volumes on I-95. As a way to compare the alternatives with one another and the Base PD&E Concept, the I-95 general use lane (GUL) traffic north and south of SW 10th Street is provided in **Table 2**. Additionally, the sum of the SW 10th Street local lane traffic at each of the three locations shown on **Figures 1 through 3** is also provided in **Table 2**. The scenarios were then ranked from 9 (worst) to 1 (best) based on these two criteria:

- The impacts to SW 10th local have been always major concern to City of Deerfield Beach and locals. An alternative with higher local traffic (higher percentage compared to Base PD&E) indicates unacceptable/degraded operations of major intersections (LOS F)
- I-95 operations is major concern to D4. Higher volumes on the I-95 GULs are undesirable and would suggest degraded operations compared to the Base PD&E. Per lane volumes exceeding LOS D indicates fatal flow alternative and highlighted in red.

Table 2: SW 10th Street Connector Intermediate Access Scenarios Evaluation

Evaluated Scenarios (2040 AM)	PD&E Base Full Access between Turnpike and I-95	Full Access between Turnpike and I-95			No Access between Turnpike and I-95			Partial Access between Turnpike and I-95		
		(A1) EL Only	(A2) EL & GUL	(A3) GUL Only	(B1) EL Only	(B2) EL & GUL	(B3) GUL Only	(C1) EL Only	(C2) EL & GUL	(C3) GUL Only
I-95 GUL NB North of Hillsboro Blvd (vphpl)*	1,730	1,700	1,820	2,150	1,800	1,830	2,220	1,770	1,820	2,130
I-95 GUL NB South of SW 10th (vphpl)*	1,830	1,780	1,800	1,830	1,890	1,820	1,920	1,840	1,790	1,820
Ranking → (Based on higher/worst volume value per lane for the I-95 NB locations)		1	3	8	6	4	9	5	2	7
SW 10th Local Traffic**	11,720	11,770	10,530	11,110	17,300	13,970	15,470	13,650	11,850	12,840
	%	100%	90%	95%	148%	119%	132%	116%	101%	110%
Rank (Based on % of the Base PD&E concept)		3	1	2	9	7	8	6	4	5

* Red indicates I-95 mainline volumes per lane exceeds LOS D target

** SW 10th local lane volumes for both directions at three locations: west of Powerline Road, west of Military Trail, and west of Newport Center Drive.

The findings based on the traffic forecast comparison can be summarized as follows:

- Scenarios A3, B3, and C3 have forecasted traffic volumes on the I-95 GULs that exceed the target Level of Service (LOS) D threshold (based on the FDOT Generalized Service Volume tables) north of the Hillsboro Boulevard interchange.
- Scenarios B1, B2, and B3 have traffic volumes on the SW 10th Street local lanes that are 19 to 48 percent higher than the Base PD&E Concept. This will result in degraded level of service conditions compared to the Base PD&E Concept. Correspondingly, the SW 10th Street Connector will be underutilized, with peak directional volumes in the range of 1,830 vehicles per hour (vph) to 2,220 vph.
- Scenarios A1, B1, and C1 maintain the Base PD&E Concept between the SW 10th Street Connector and I-95 express lanes and are less preferred by the City of Deerfield Beach. The Scenario A1 traffic volume for the egress east of Military Trail is 1,780 vph, which is approaching the practical capacity of a single lane ramp. The Scenario B1 and C1 traffic volumes on the SW 10th Street local lanes are 48 and 16 percent higher than the Base PD&E Concept, respectively. As a result, traffic operations along SW 10th local lanes will be degraded compared to the Base PD&E Concept.

Based on the findings from the traffic volume comparisons and considering the project goal to achieve concurrence from the City of Deerfield Beach, the project team determined that Scenarios A2 and C2 should move forward for further operational analysis by the PD&E teams. Scenario D2 was introduced as a hybrid option and presented to the City of Deerfield Beach. It provides an eastbound ingress and westbound egress serving Powerline Road to and from the east. There are no access ramps serving Newport Center Drive. The three scenarios (A2, C2, and D2) are illustrated on **Figure 4**. The 2040 AM and PM SW 10th Street Connector volumes between the Turnpike and I-95 for Scenarios A2, C2, and D2 are presented on **Figures 5 and 6**, respectively. The D2 scenario local SW 10th traffic is much higher than options A2 and C2 including the Base PD&E option by at least 7 percent leading to undesirable operations (i.e, Military trail intersection will operate at LOS F), while SW 10th Street Connector will be underutilized. Accordingly, this scenario will not be analyzed further, though the traffic volumes are provided herein for documentation purposes only.

Detailed 2020 and 2040 AM and PM turning movement projections are provided in **Appendix D** for the Base PD&E, A2, and C2 concepts for an expanded analysis area that includes intersections north and south of SW 10th Street and the interchanges at Hillsboro Boulevard and Sample Road. The 2020 and 2040 Annual Average Daily Traffic (AADT) was re-estimated for A2 and C2 concepts and these volumes are provided in **Appendix E**. The No Action alternative 2020 and 2040 AADT and AM/PM turns are provided in **Appendix F**.

Figure 4. I-95 Express and I-95 General Use Lane Connections (Scenarios A2, C2, and D2)

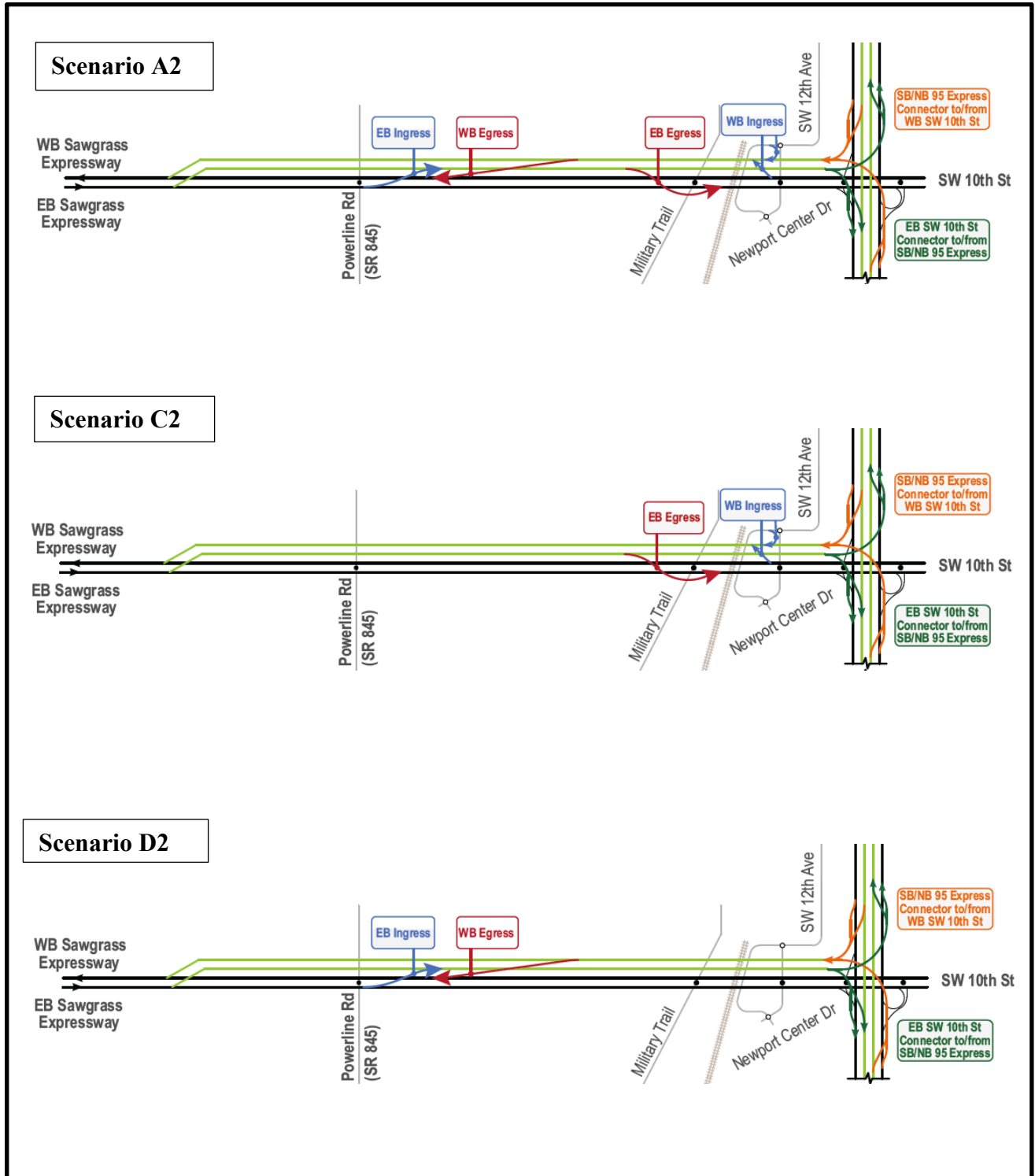


Figure 5: Year 2040 AM Peak Hour Traffic Forecasts

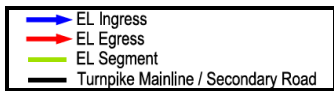
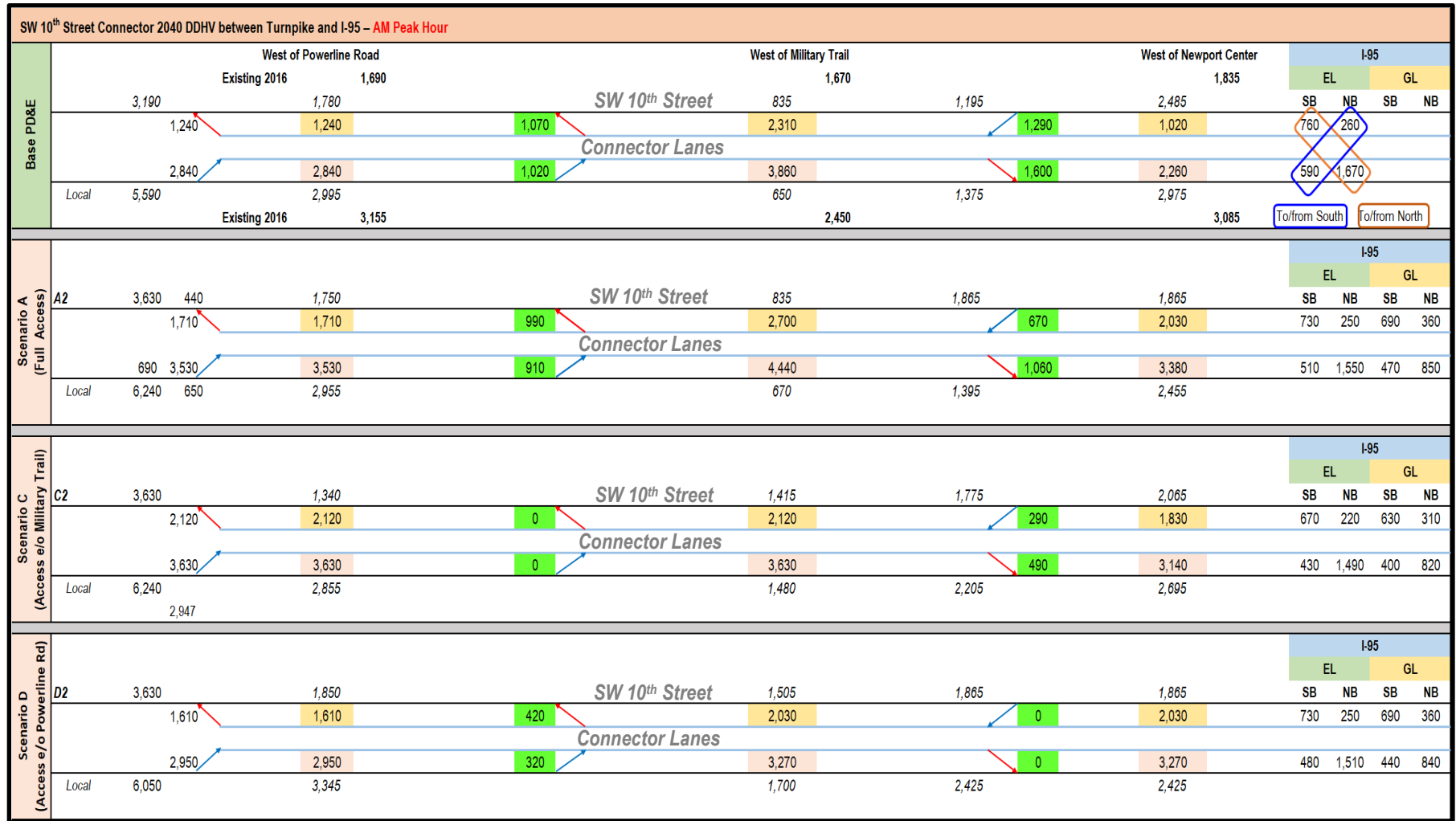
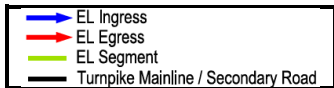


Figure 6: Year 2040 PM Peak Hour Traffic Forecasts

SW 10 th Street Connector 2040 DDHV between Turnpike and I-95 – PM Peak Hour									
Base PD&E	West of Powerline Road		West of Military Trail		West of Newport Center		I-95		
	Existing 2016	3,260	2,590		2,795	EL	GL	SB	NB
	5,590	2,890	1,240	3,550	3,145			SB	NB
	2,840	2,840	710	1,770	1,780			1,130	650
	1,240	1,240	1,230	1,350	1,120			220	900
Local	3,190	1,885	835	2,500				To/from South	To/from North
	Existing 2016	1,665	1,890	1,950					
Scenario A (Full Access)	West of Powerline Road		West of Military Trail		West of Newport Center		I-95		
A2	6,240	2,850	1,240	4,150	2,545			SB	NB
Local	3,530	3,530	620	1,170	2,980			1,030	550
	1,710	1,710	1,150	750	2,110			210	810
	3,630	1,855	835	1,900				420	670
Scenario C (Access e/o Military Trail)	West of Powerline Road		West of Military Trail		West of Newport Center		I-95		
C2	6,240	3,050	2,060	3,330	2,795			SB	NB
	3,330	3,330	0	600	2,730			980	490
	2,280	2,280	0	320	1,960			180	770
Local	3,630	1,285	1,415	2,050				390	620
Scenario D (Access e/o Powerline Rd)	West of Powerline Road		West of Military Trail		West of Newport Center		I-95		
D2	6,050	3,650	2,470	2,780	2,605			SB	NB
	2,540	2,540	240	0	2,780			970	510
	1,580	1,580	530	0	2,110			210	810
Local	3,630	1,985	1,605	1,920				420	670

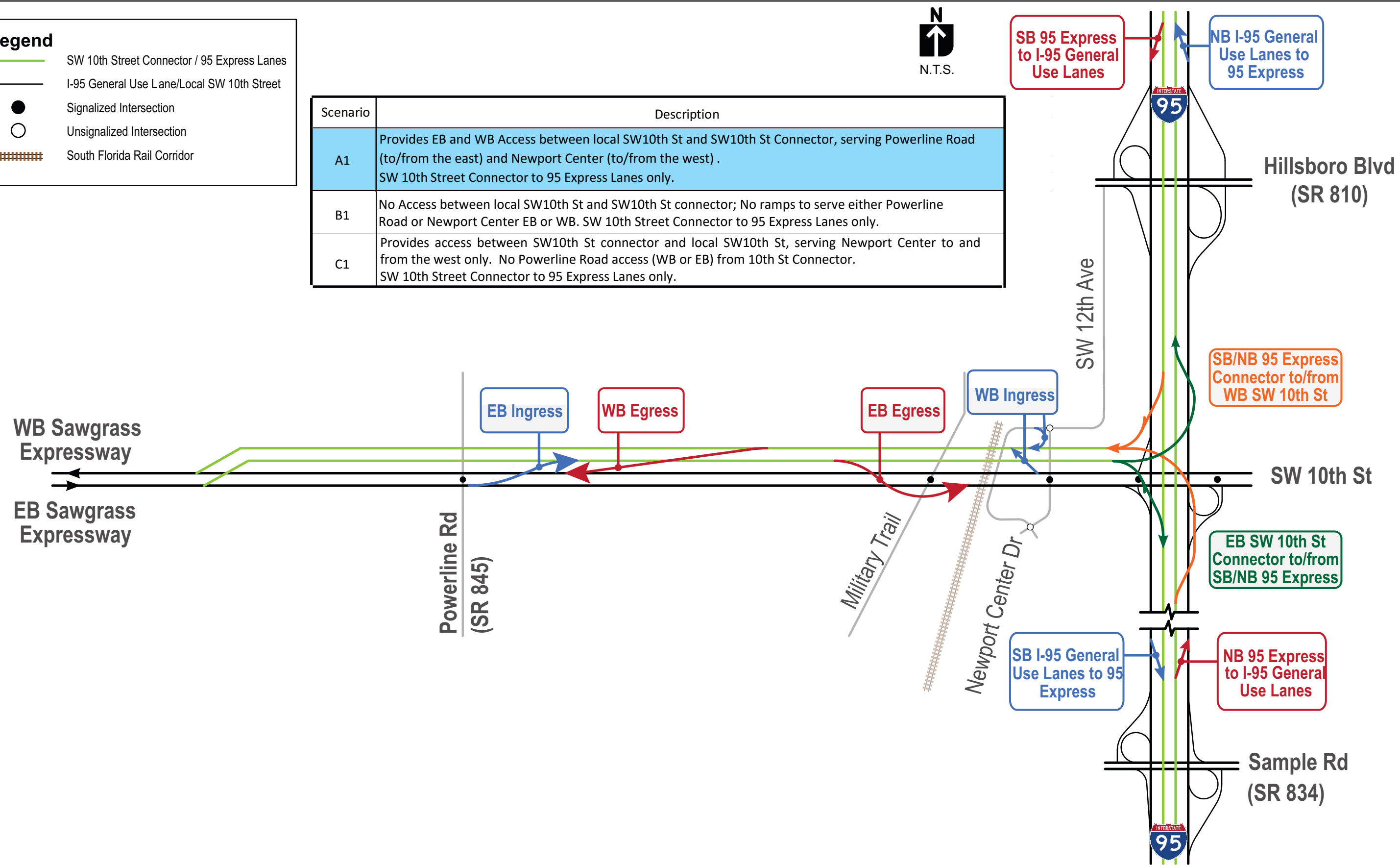


APPENDIX A

Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
- Signalized Intersection
- Unsignalized Intersection
- ##### South Florida Rail Corridor

Scenario	Description
A1	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west). SW 10th Street Connector to 95 Express Lanes only.
B1	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 Express Lanes only.
C1	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
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C1	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



WB Sawgrass Expressway
 EB Sawgrass Expressway

Powerline Rd
 (SR 845)

Military Trail

Newport Center Dr

SW 12th Ave

Hillsboro Blvd
 (SR 810)

SW 10th St

EB SW 10th St
 Connector to/from
 SB/NB 95 Express

Sample Rd
 (SR 834)

SB I-95 General
 Use Lanes to
 95 Express

NB 95 Express
 to I-95 General
 Use Lanes

SB 95 Express
 to I-95 General
 Use Lanes

NB I-95 General
 Use Lanes to
 95 Express

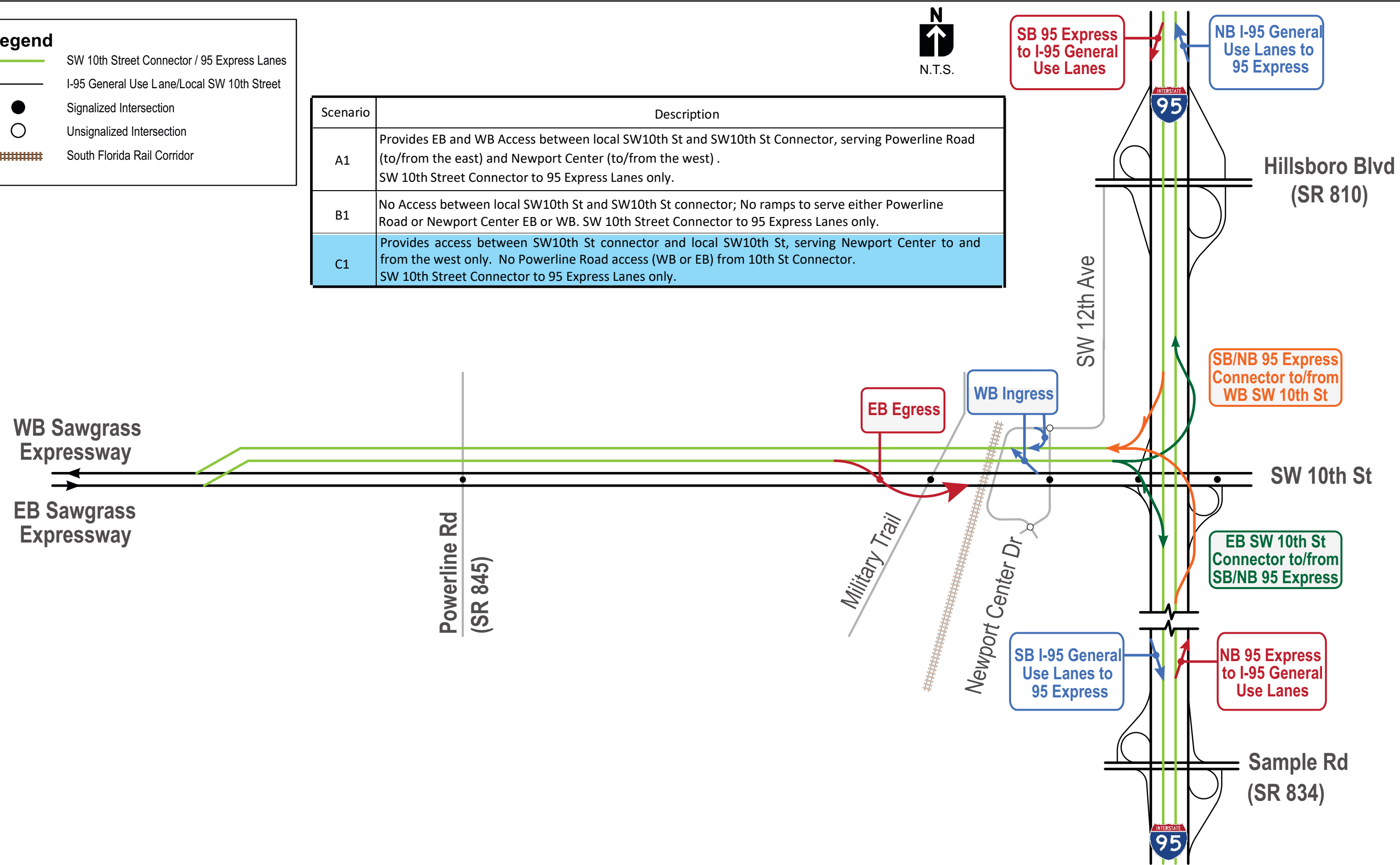
SB/NB 95 Express
 Connector to/from
 WB SW 10th St



Legend

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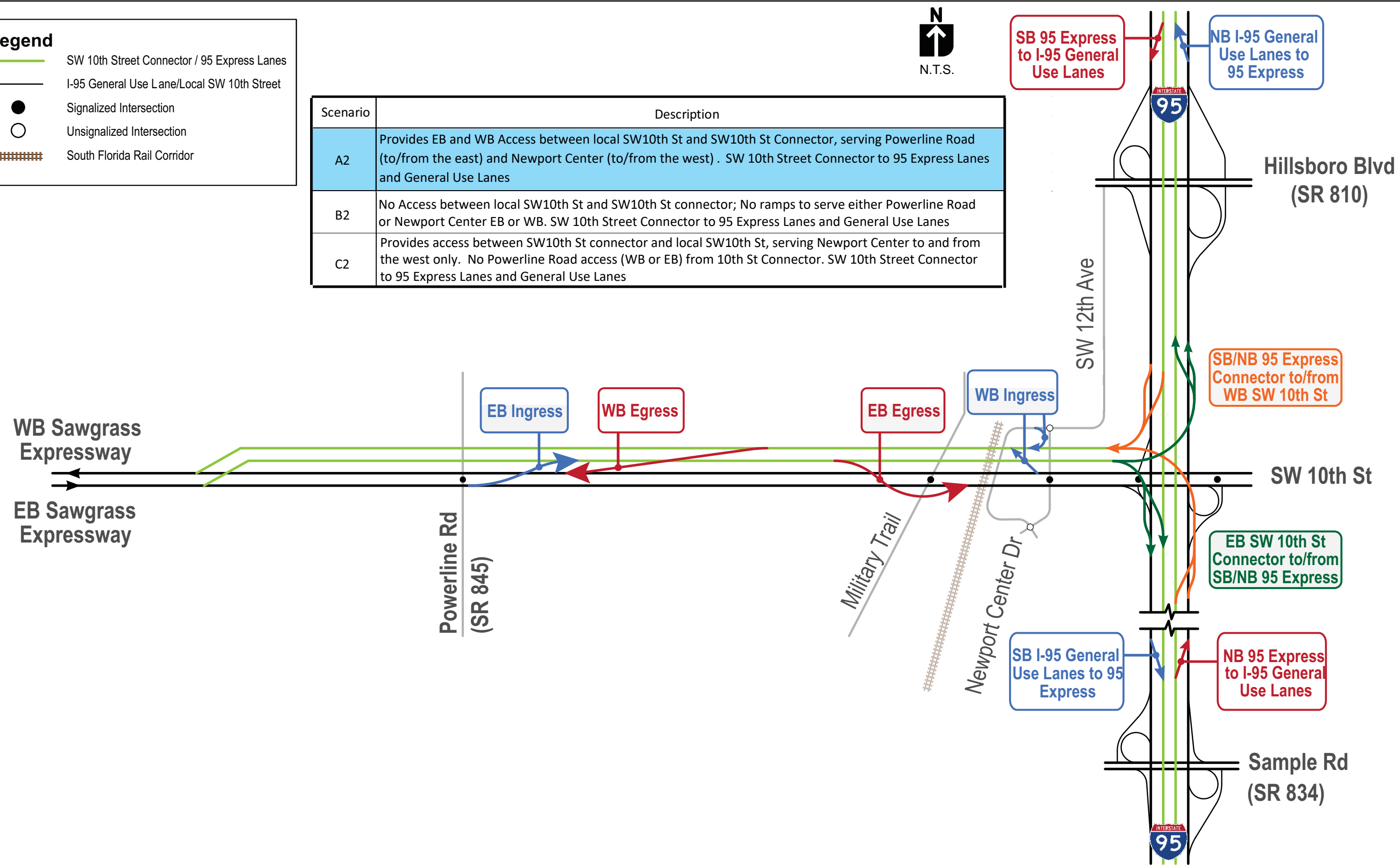
Scenario	Description
A1	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 Express Lanes only.
B1	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 Express Lanes only.
C1	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



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- ##### South Florida Rail Corridor

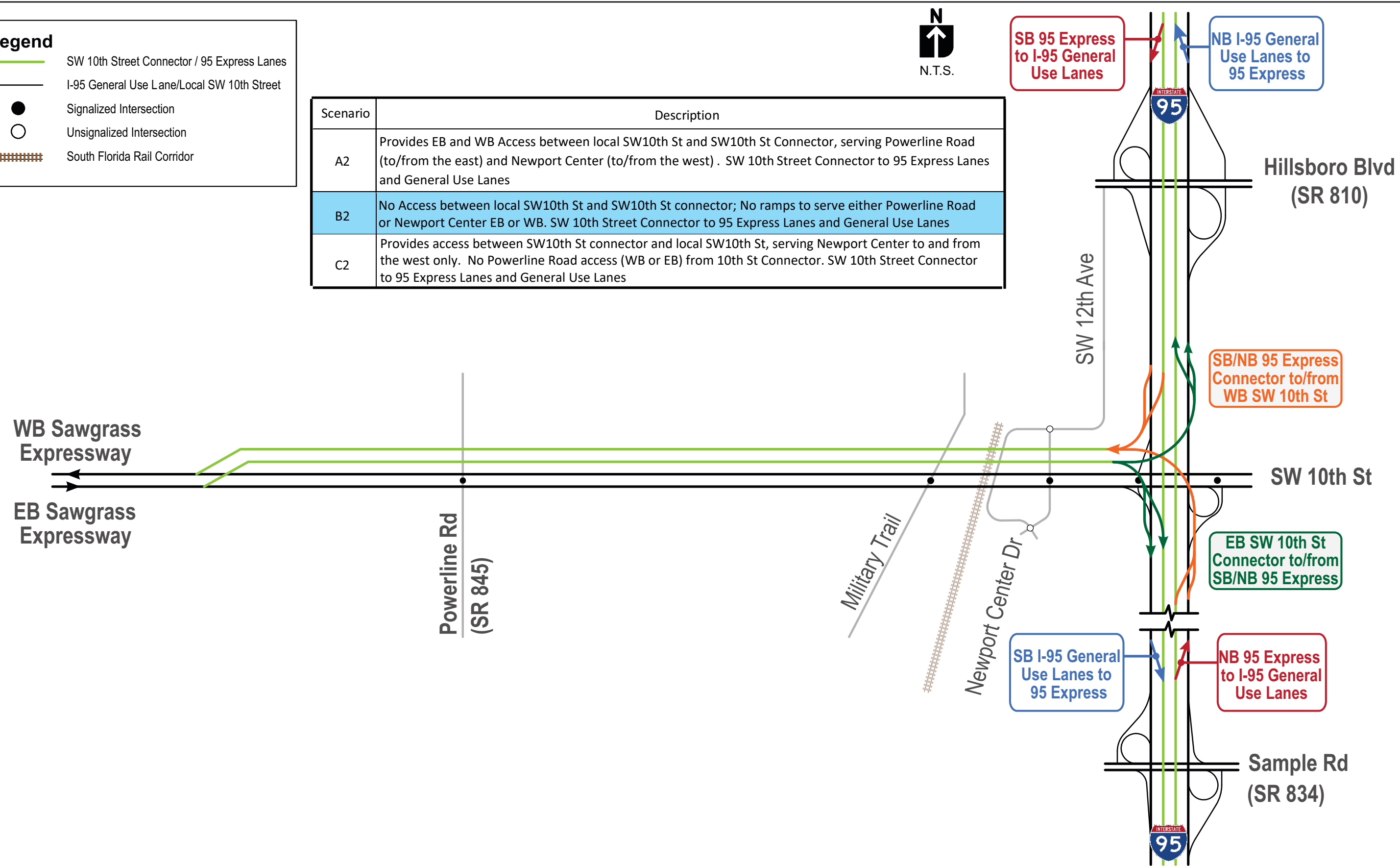
Scenario	Description
A2	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 Express Lanes and General Use Lanes
B2	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 Express Lanes and General Use Lanes
C2	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector. SW 10th Street Connector to 95 Express Lanes and General Use Lanes



Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
- Signalized Intersection
- Unsignalized Intersection
- ##### South Florida Rail Corridor

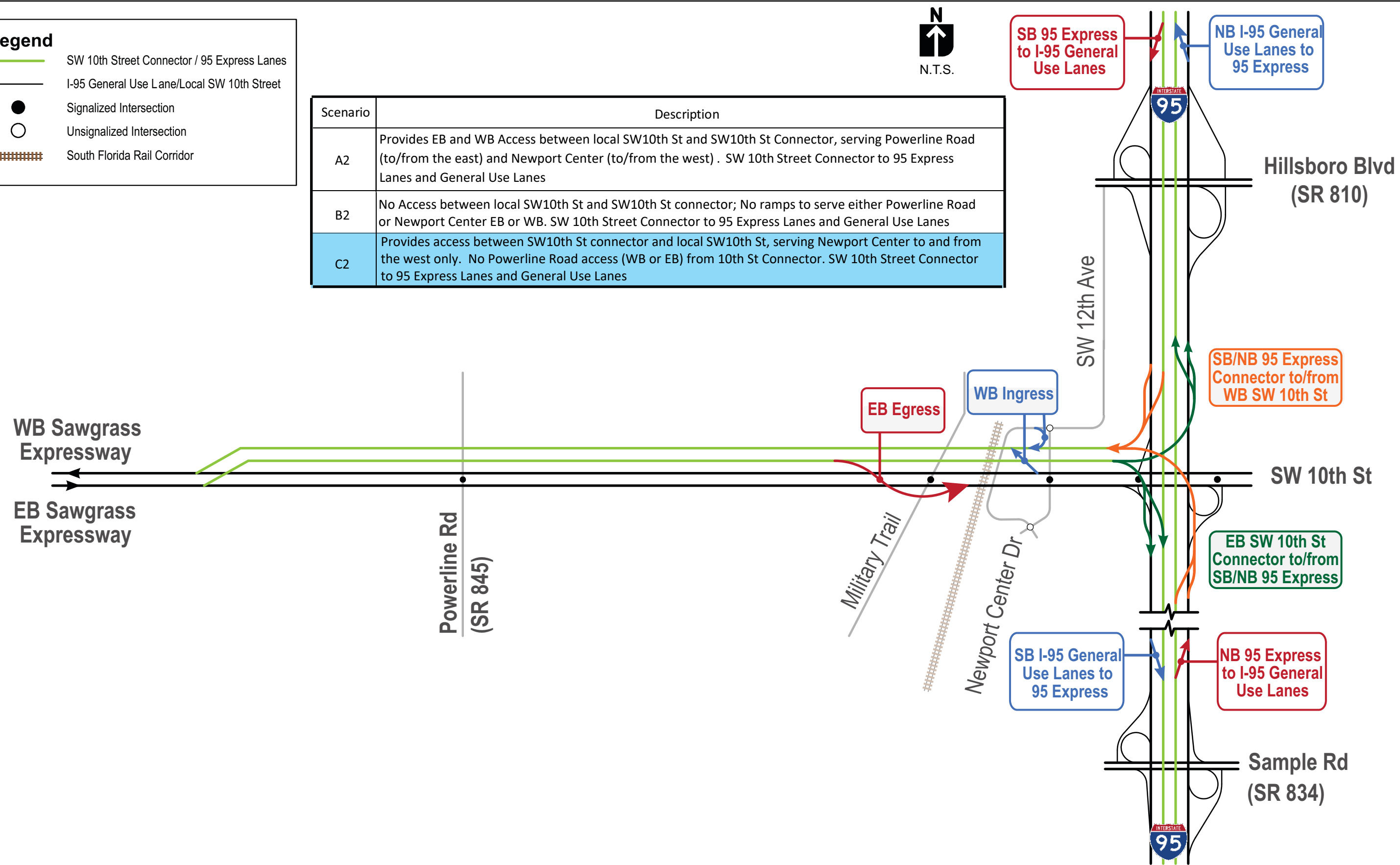
Scenario	Description
A2	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 Express Lanes and General Use Lanes
B2	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 Express Lanes and General Use Lanes
C2	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector. SW 10th Street Connector to 95 Express Lanes and General Use Lanes



Legend

- SW 10th Street Connector / 95 Express Lanes
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- Unsignalized Intersection
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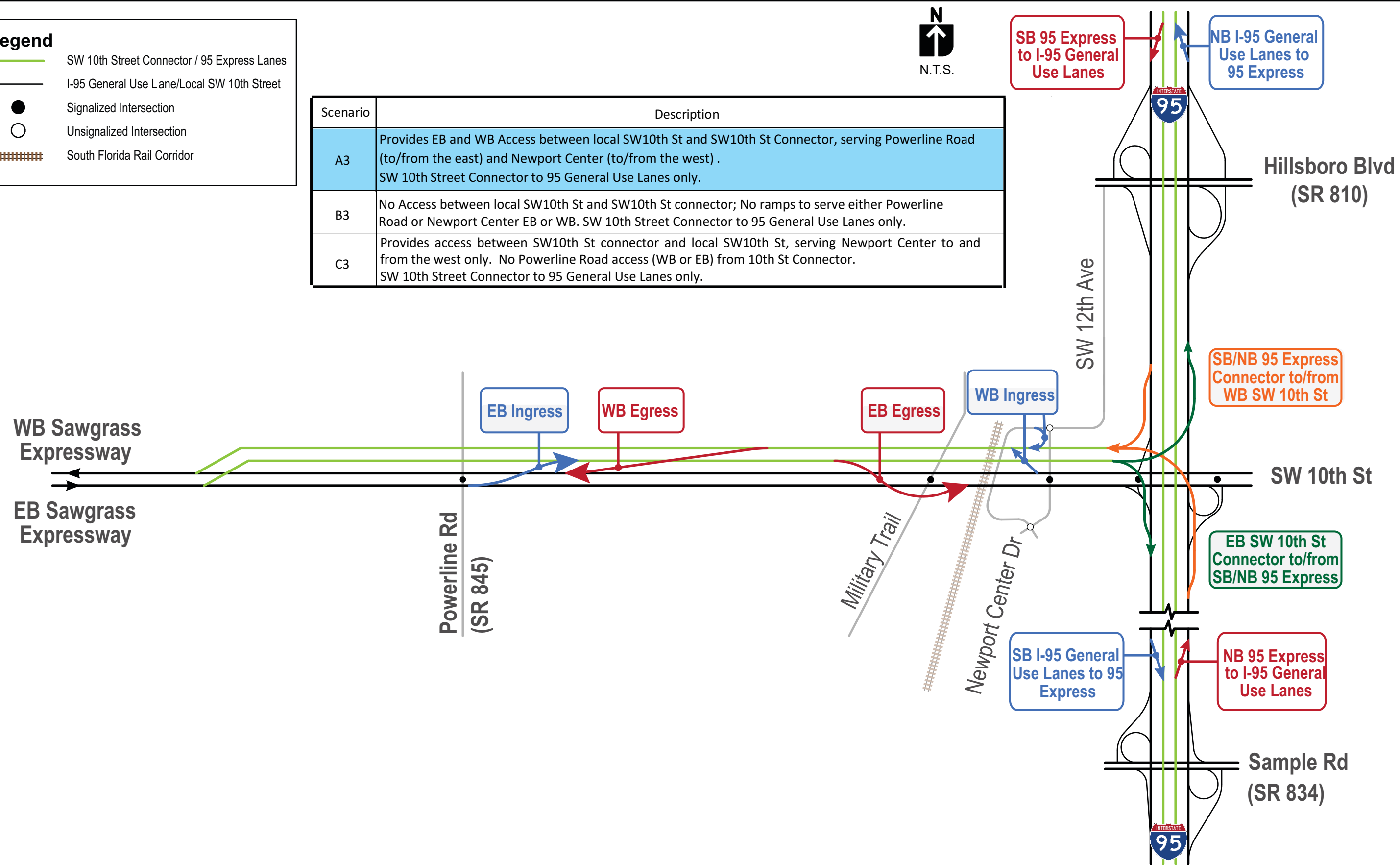
Scenario	Description
A2	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 Express Lanes and General Use Lanes
B2	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 Express Lanes and General Use Lanes
C2	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector. SW 10th Street Connector to 95 Express Lanes and General Use Lanes



Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
- Signalized Intersection
- Unsignalized Intersection
- ##### South Florida Rail Corridor

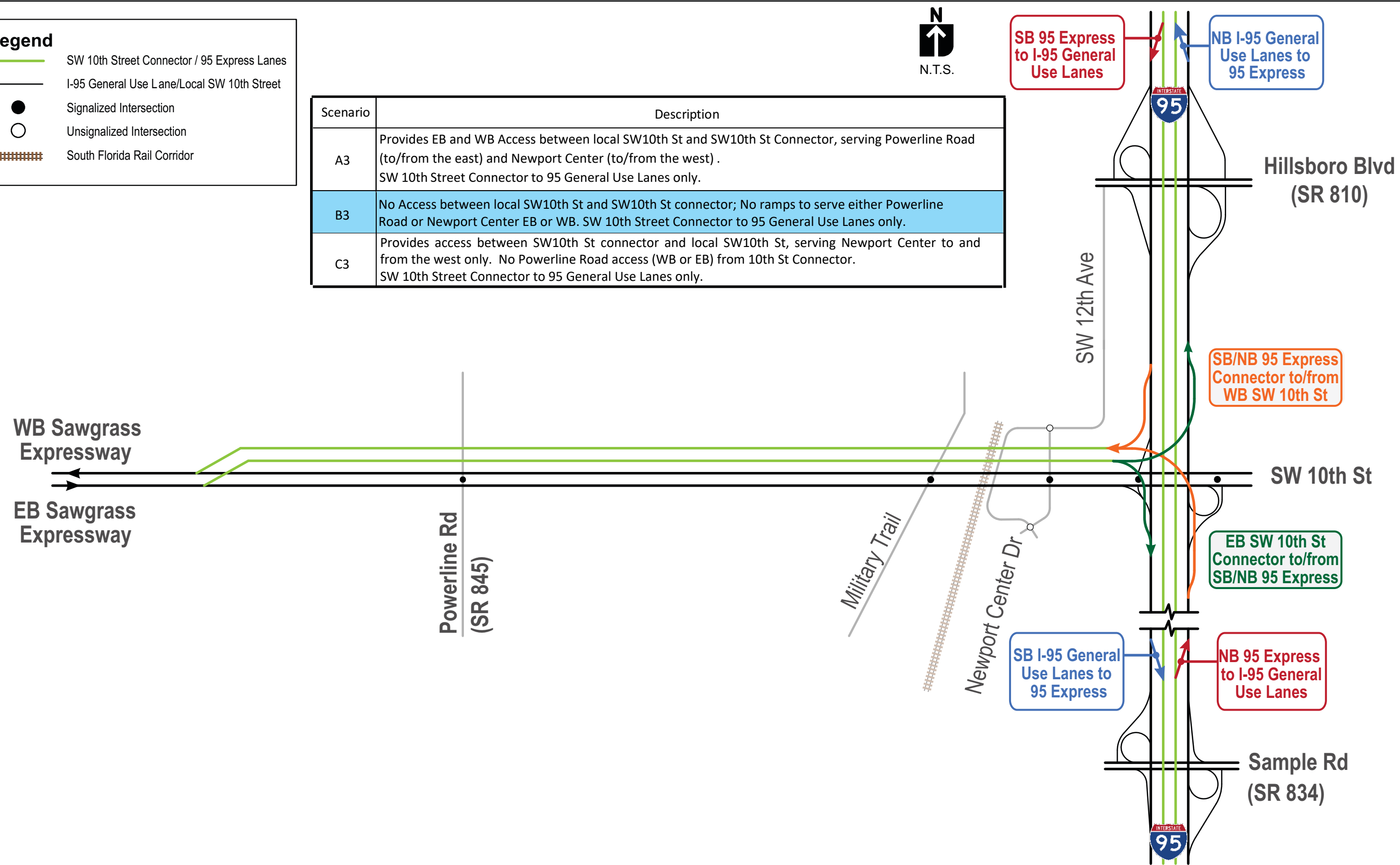
Scenario	Description
A3	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 General Use Lanes only.
B3	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 General Use Lanes only.
C3	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
- Signalized Intersection
- Unsignalized Intersection
- ##### South Florida Rail Corridor

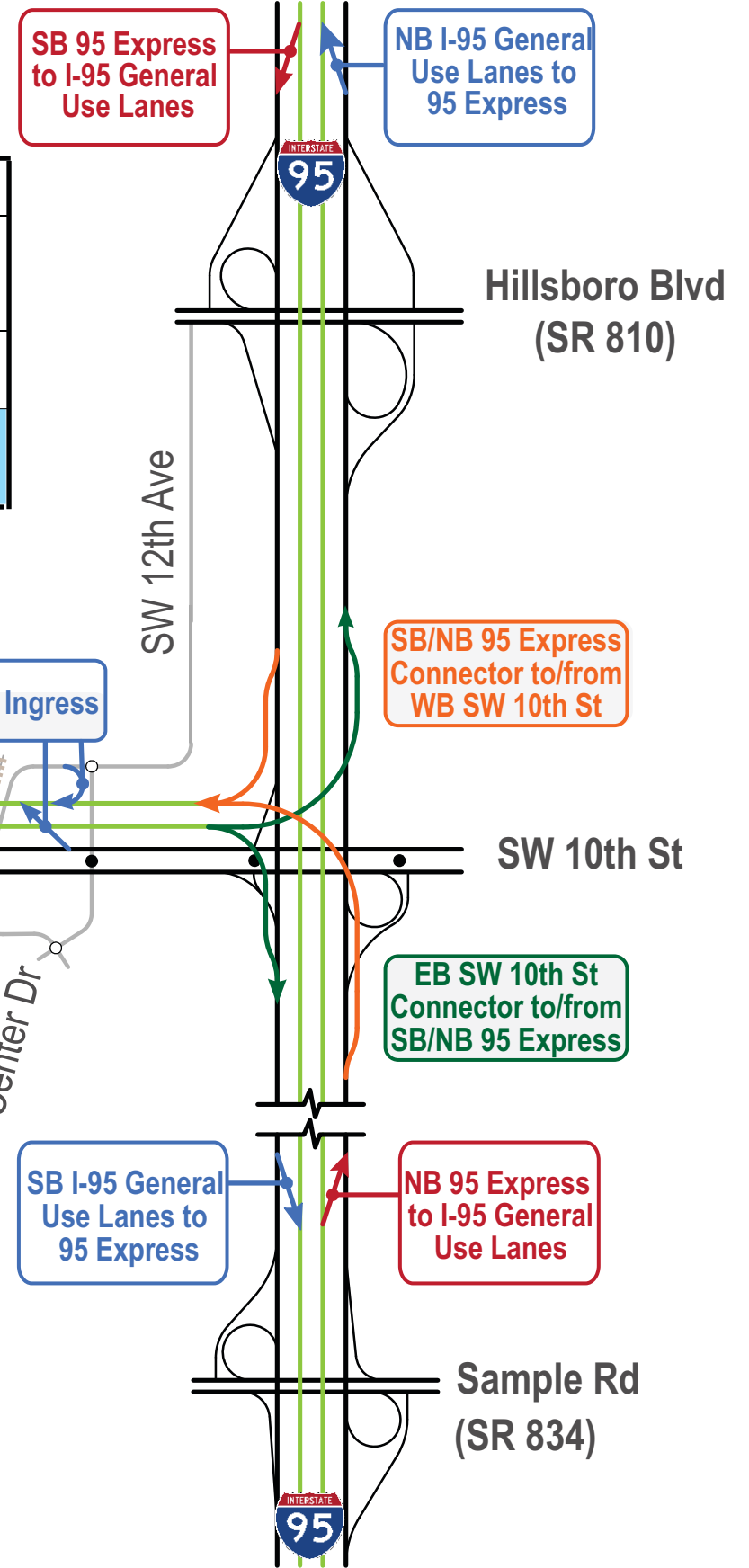
Scenario	Description
A3	Provides EB and WB Access between local SW10th St and SW10th St Connector, serving Powerline Road (to/from the east) and Newport Center (to/from the west) . SW 10th Street Connector to 95 General Use Lanes only.
B3	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 General Use Lanes only.
C3	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



Legend

- SW 10th Street Connector / 95 Express Lanes
- I-95 General Use Lane/Local SW 10th Street
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- Unsignalized Intersection
- ##### South Florida Rail Corridor

Scenario	Description
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B3	No Access between local SW10th St and SW10th St connector; No ramps to serve either Powerline Road or Newport Center EB or WB. SW 10th Street Connector to 95 General Use Lanes only.
C3	Provides access between SW10th St connector and local SW10th St, serving Newport Center to and from the west only. No Powerline Road access (WB or EB) from 10th St Connector.



WB Sawgrass Expressway
 EB Sawgrass Expressway

Powerline Rd
 (SR 845)

Military Trail

Newport Center Dr

Hillsboro Blvd
 (SR 810)

SW 10th St

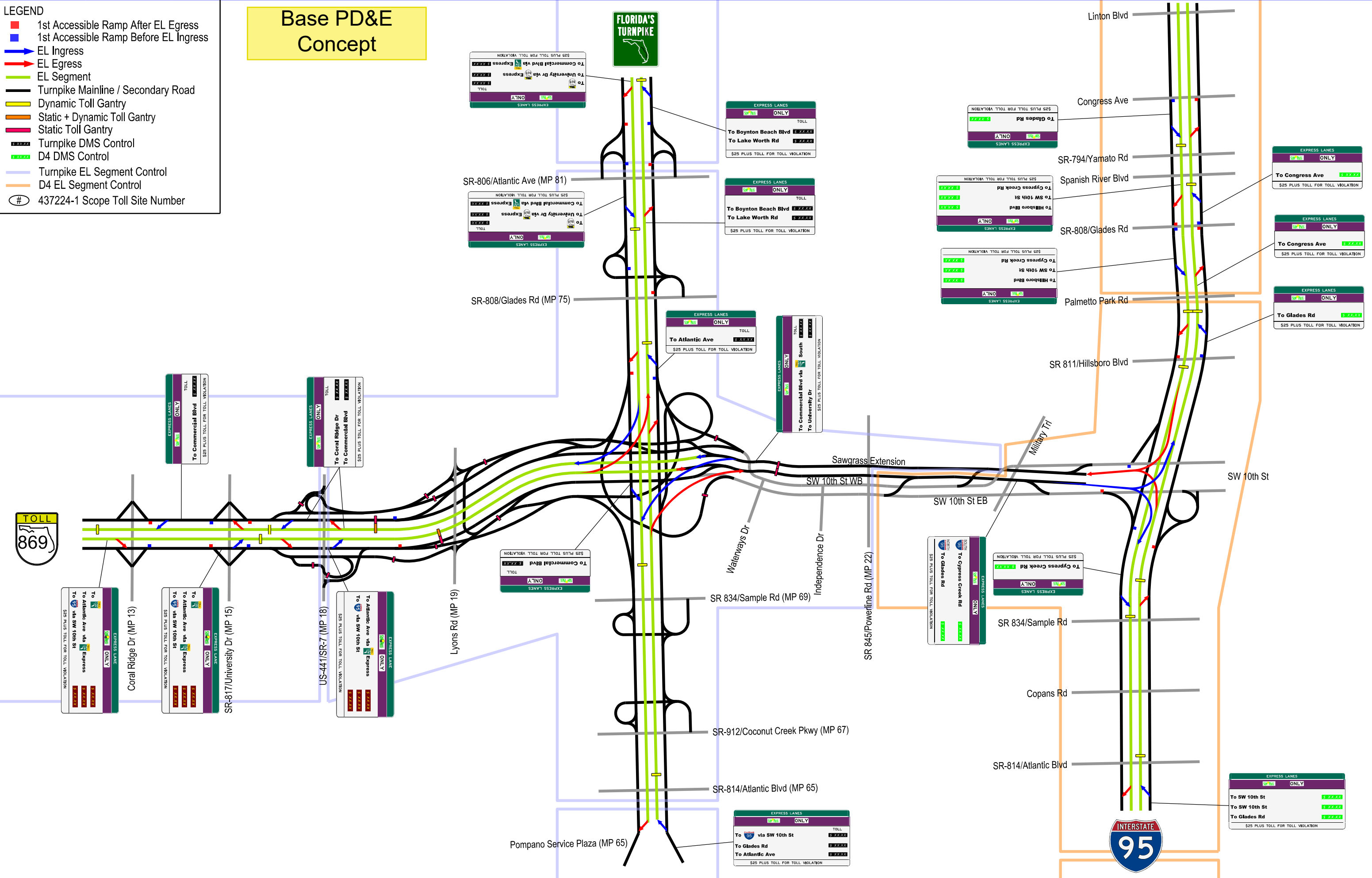
Sample Rd
 (SR 834)



APPENDIX B

- LEGEND**
- 1st Accessible Ramp After EL Egress
 - 1st Accessible Ramp Before EL Ingress
 - EL Ingress
 - EL Egress
 - EL Segment
 - Turnpike Mainline / Secondary Road
 - Dynamic Toll Gantry
 - Static + Dynamic Toll Gantry
 - Static Toll Gantry
 - XXXXX Turnpike DMS Control
 - XXXXX D4 DMS Control
 - Turnpike EL Segment Control
 - D4 EL Segment Control
 - # 437224-1 Scope Toll Site Number

Base PD&E Concept



LEGEND

- 1st Accessible Ramp After ML/EL Egress
- 1st Accessible Ramp Before ML/EL Ingress
- GUL/GTL Lane Ingress/Egress
- ML/EL Ingress
- ML/EL Egress
- S Slip Ramp Ingress/Egress Layout
- W Weave Lane Ingress/Egress Layout
- WZ Weave Zone Ingress/Egress Layout
- # Number of Lanes
- EL Segment
- ML Segment
- EL Toll Gantry
- GUL / GTL Toll Gantry
- Toll Gantry Taken Out Of Production
- Data Gantry
- Gantry With No Equipment
- Mainline GUL / GTL / Secondary Road
- XX Miles

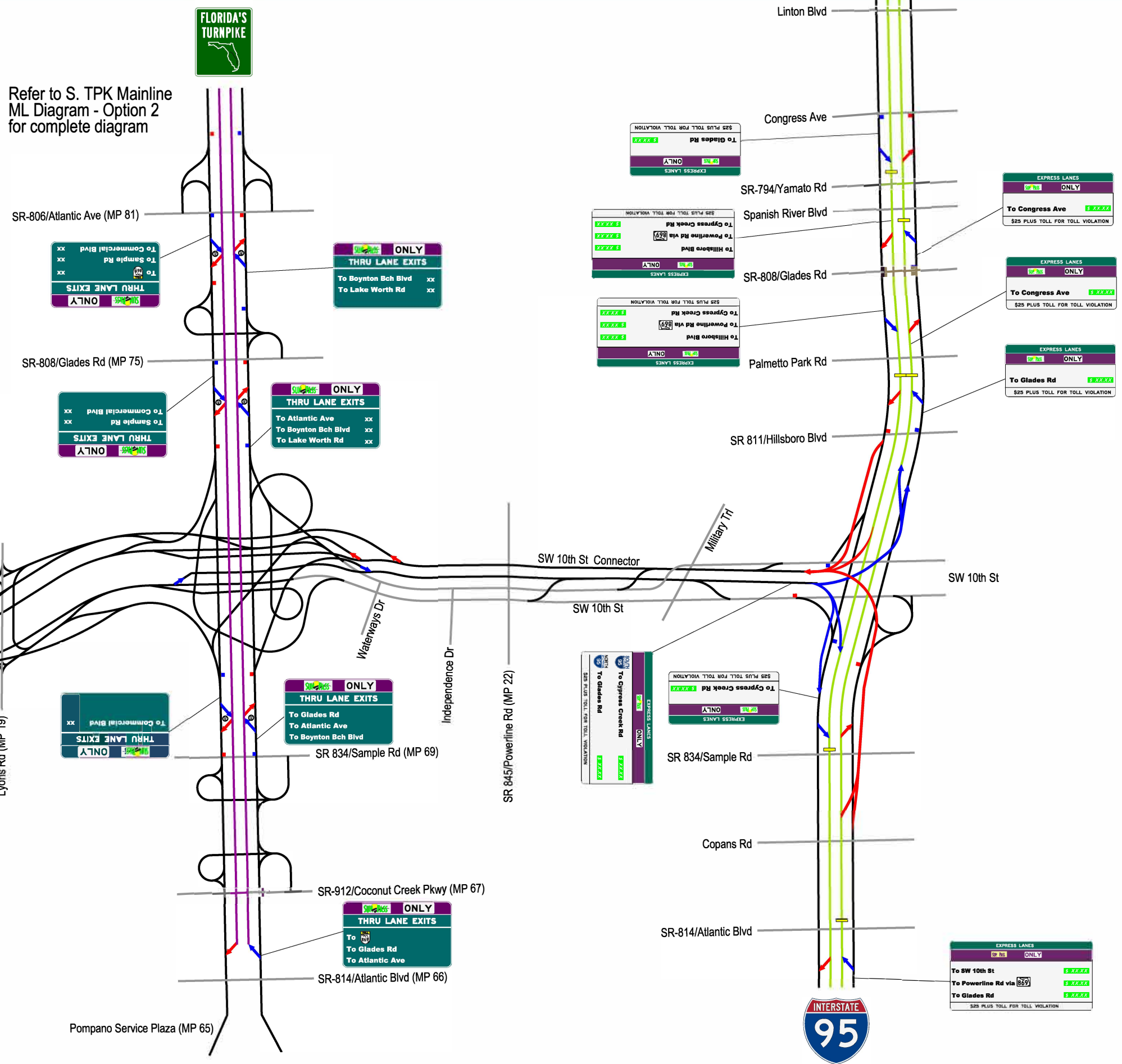
SW 10th Street Scenario A2

Refer to S. TPK Mainline ML Diagram - Option 2 for complete diagram



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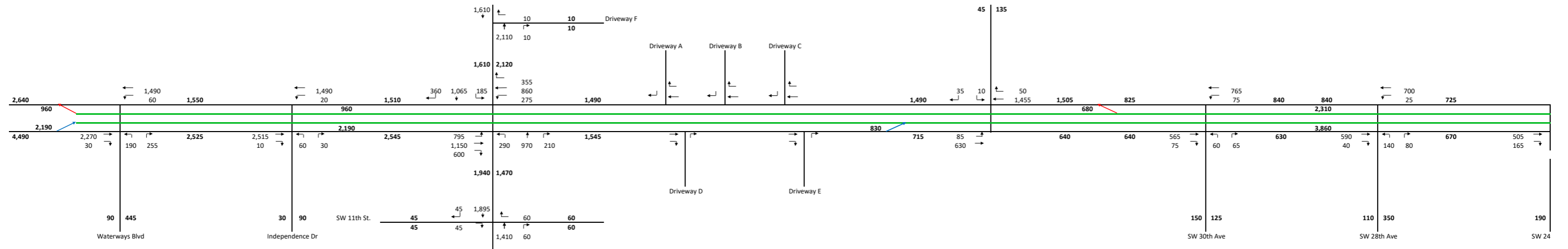
*Note: Sawgrass/TPK Interchange is under development



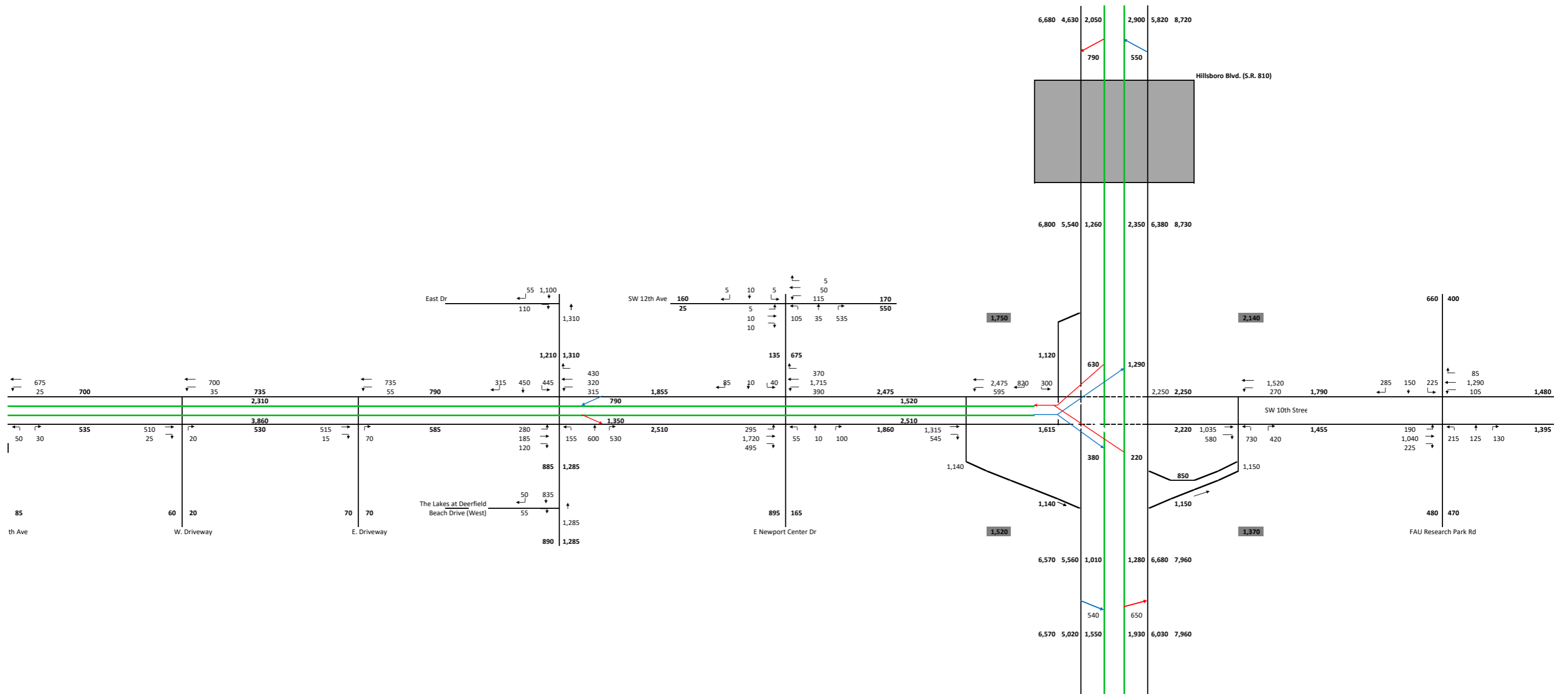
APPENDIX C

APPENDIX D

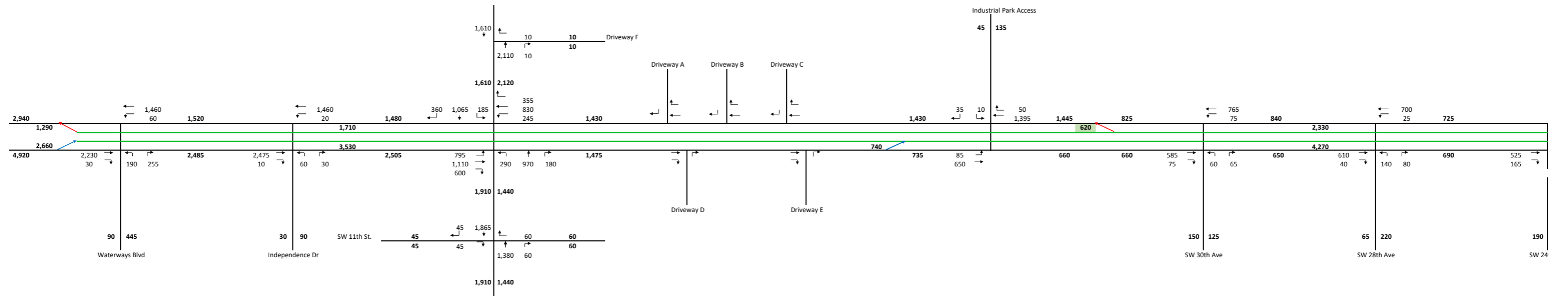
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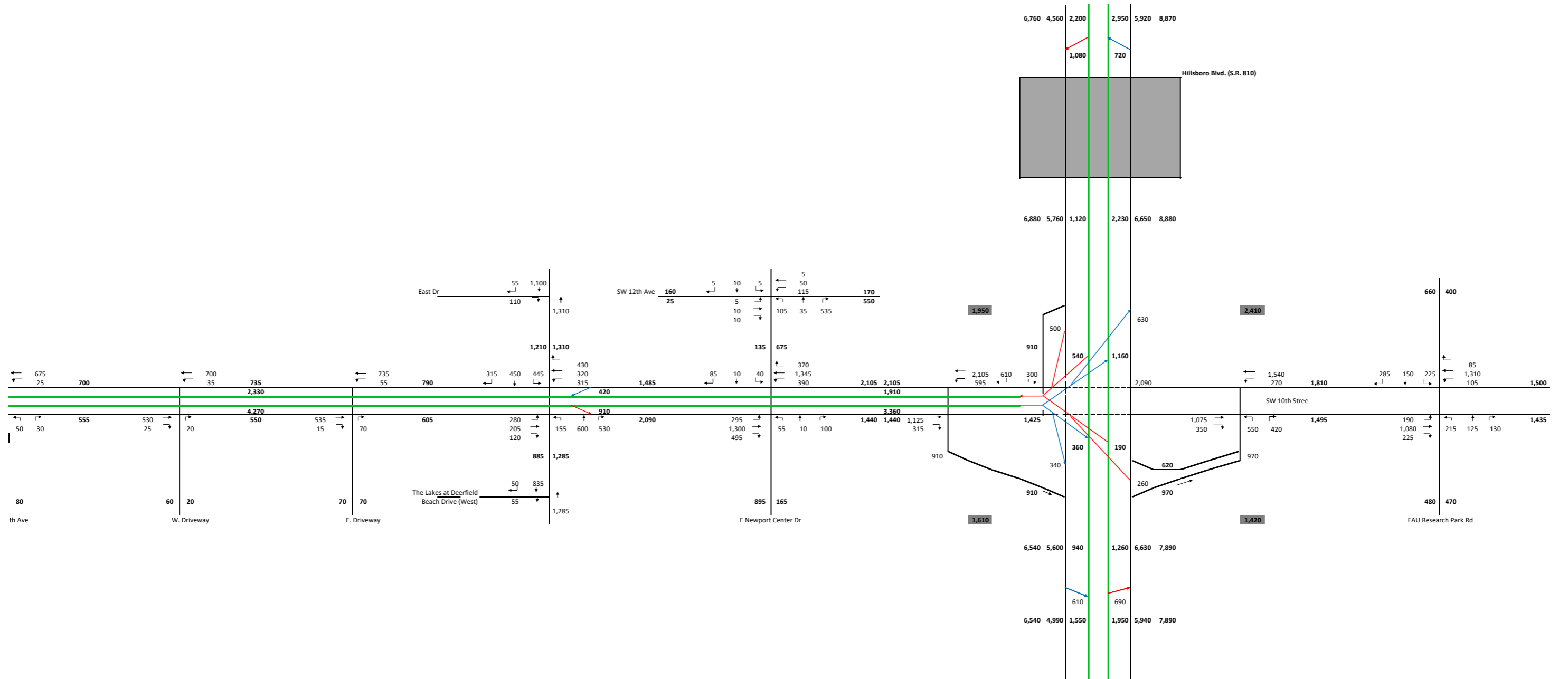
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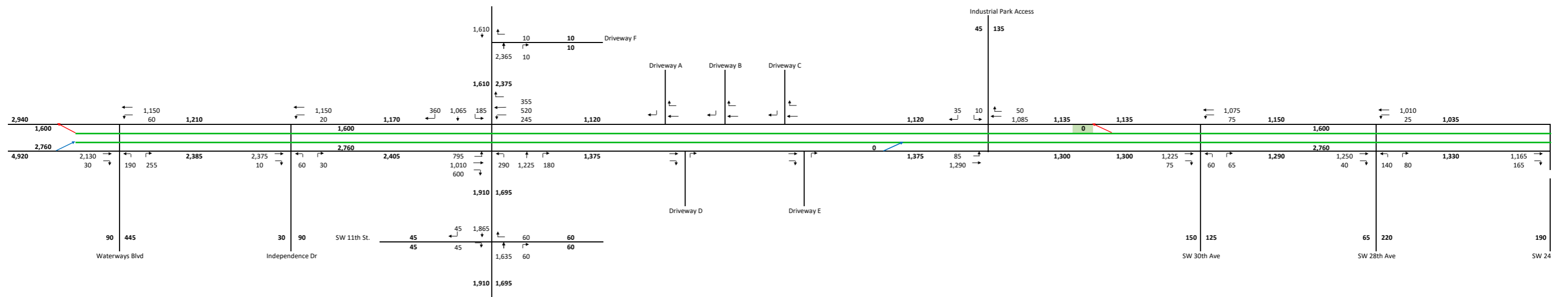
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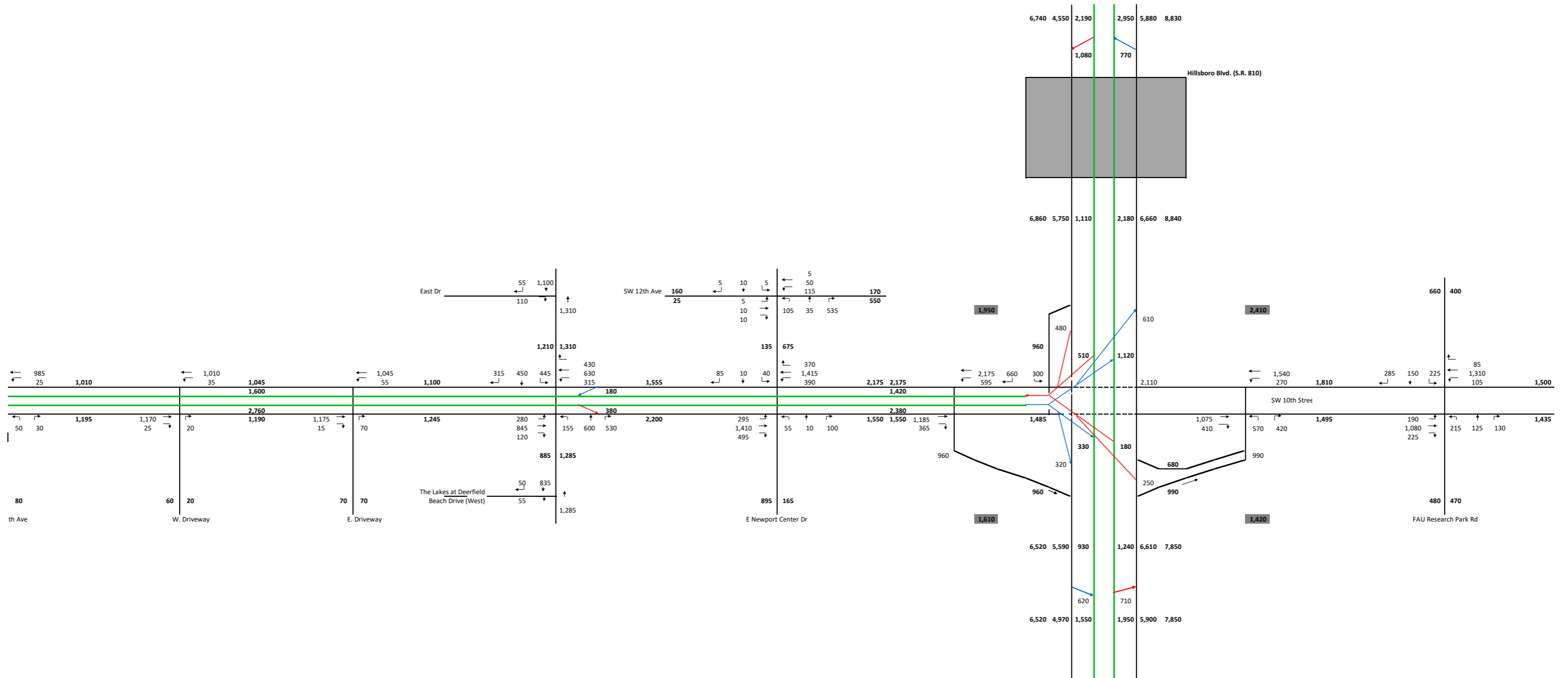
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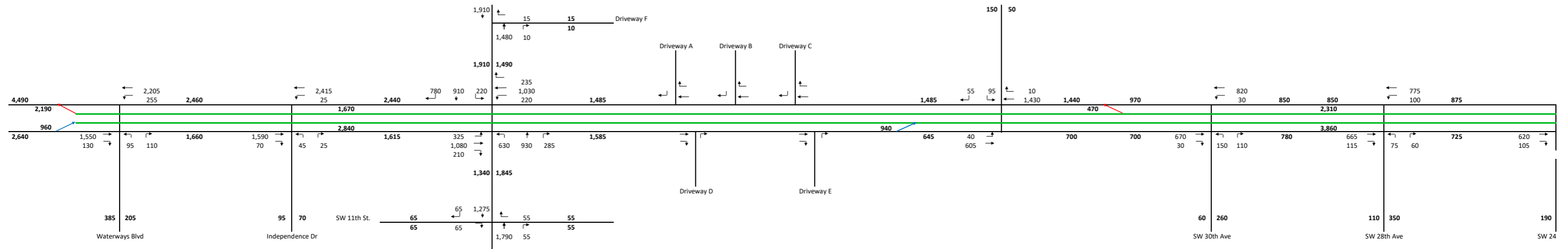
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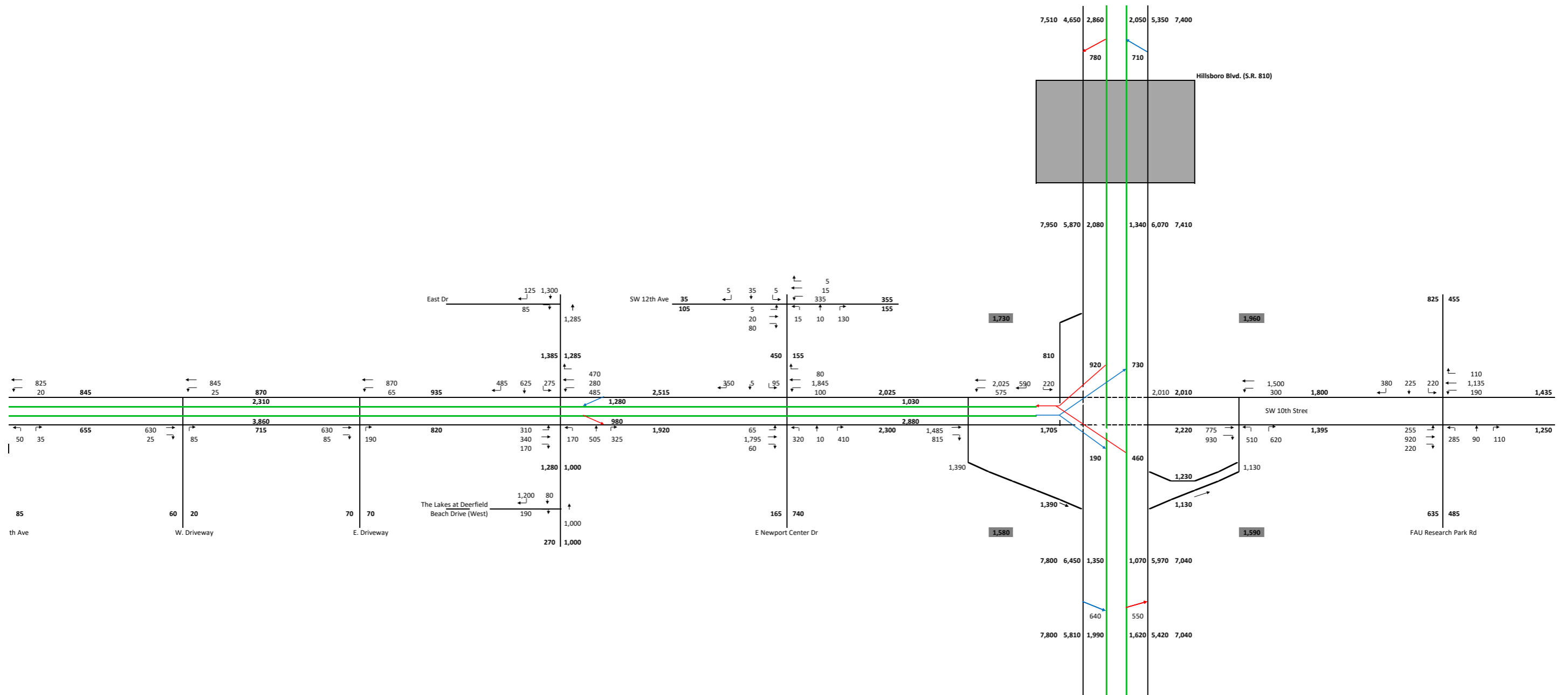
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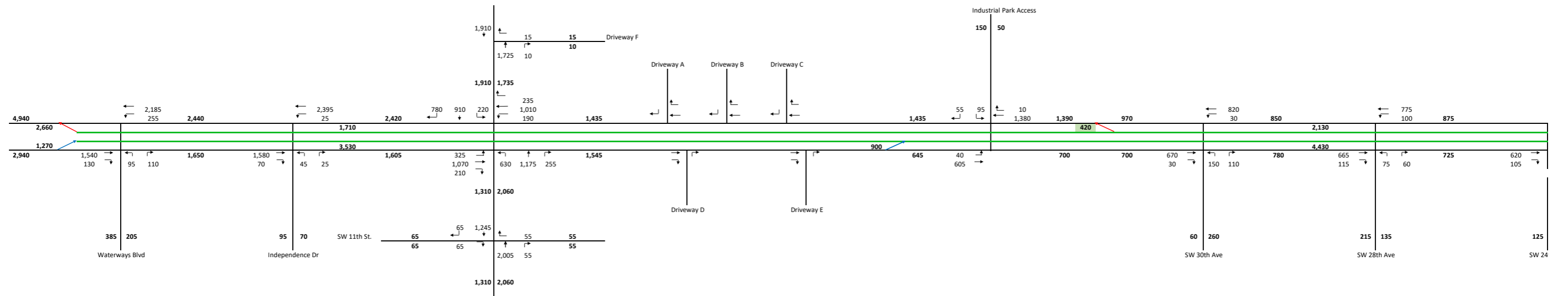
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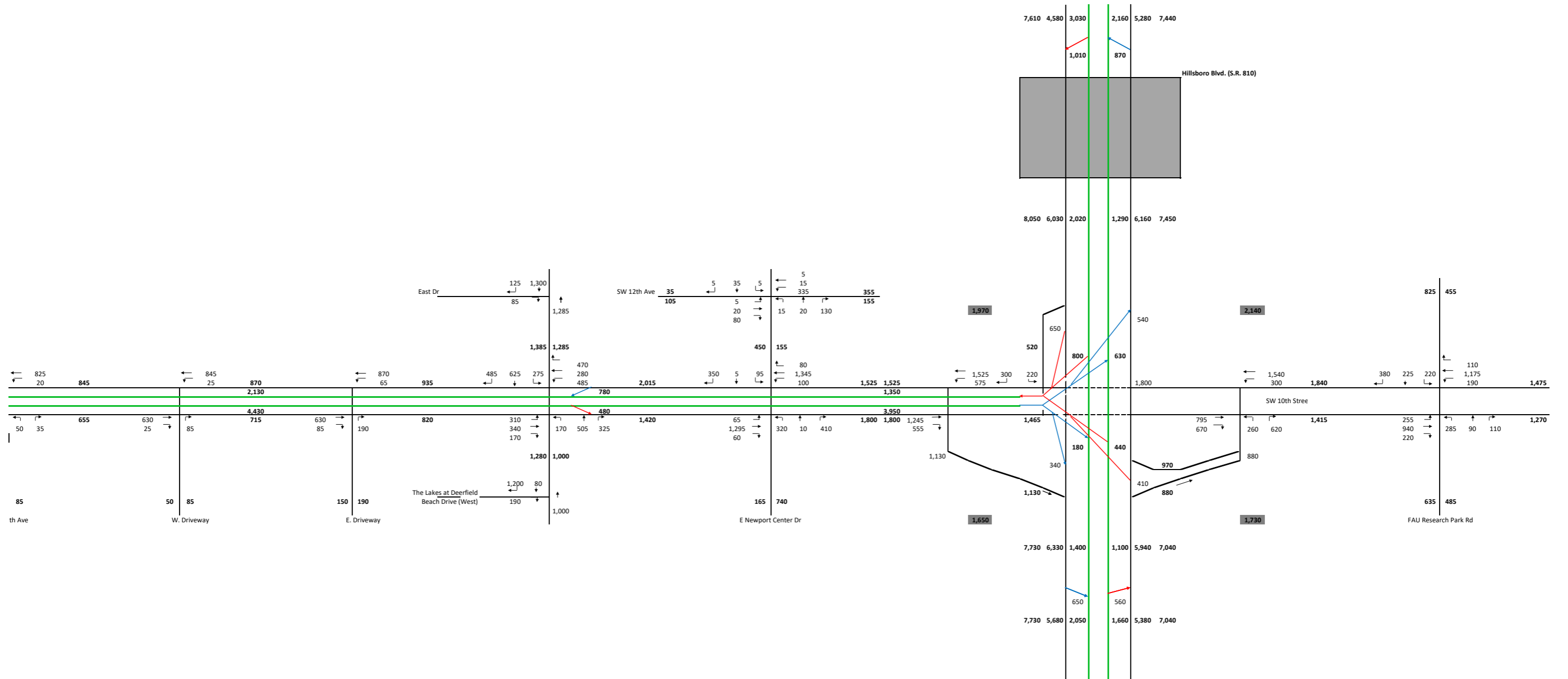
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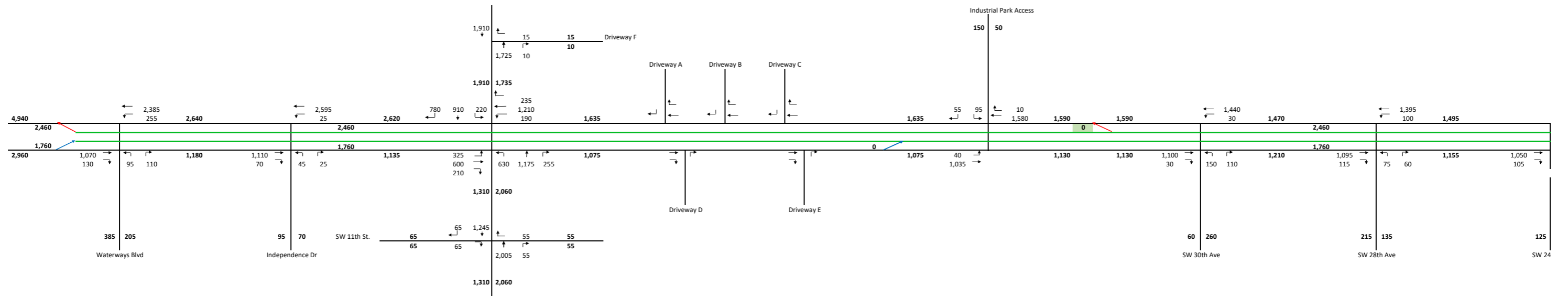
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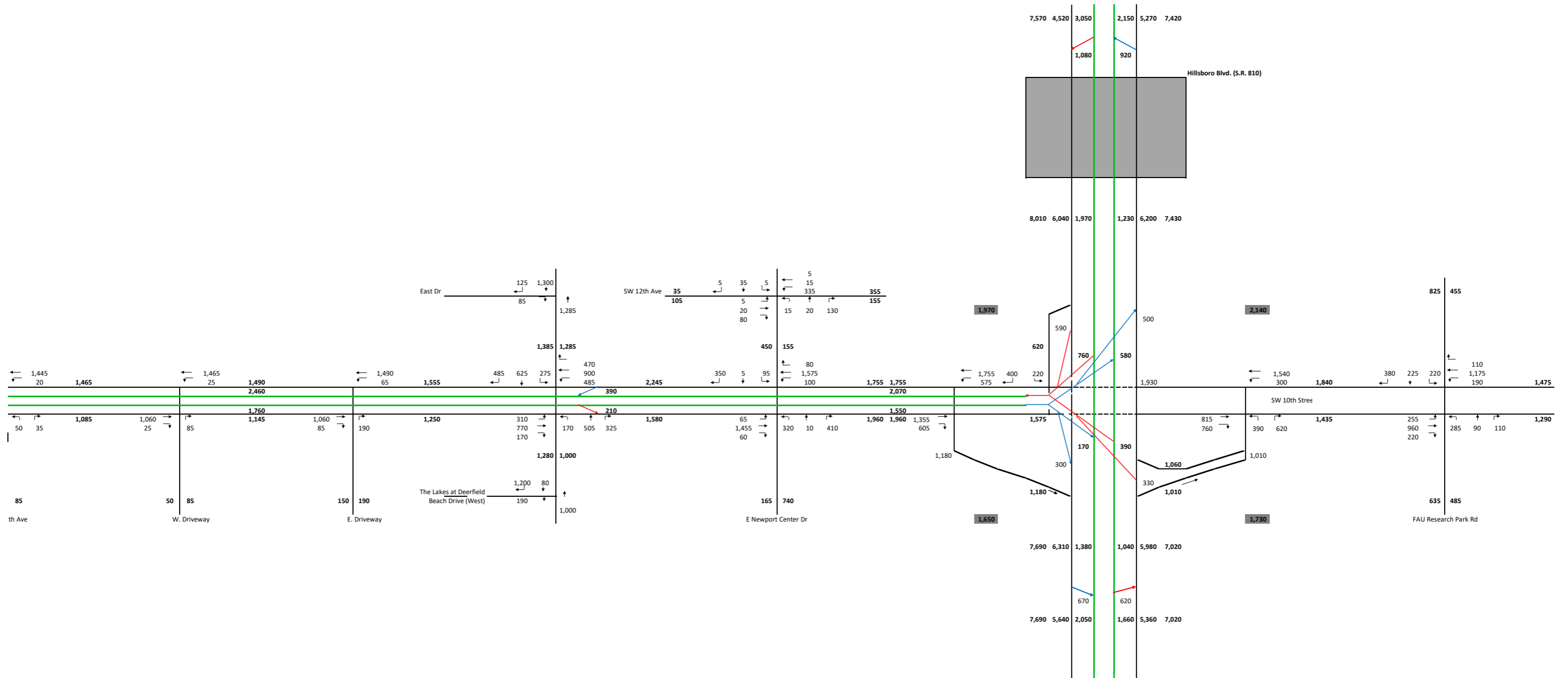
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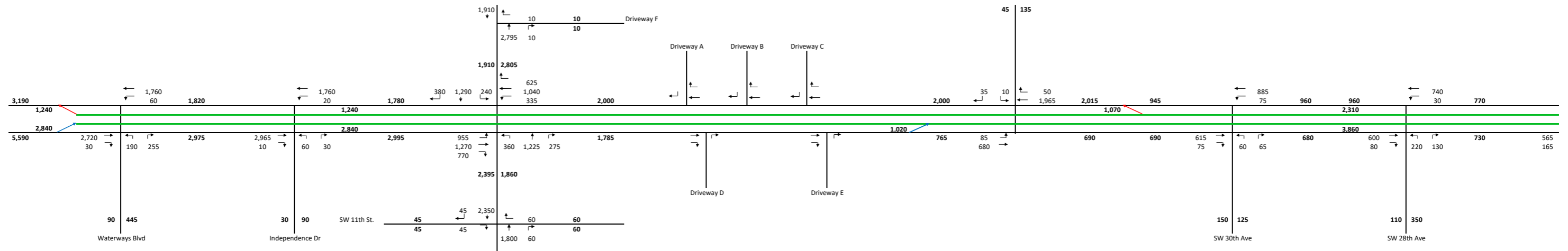
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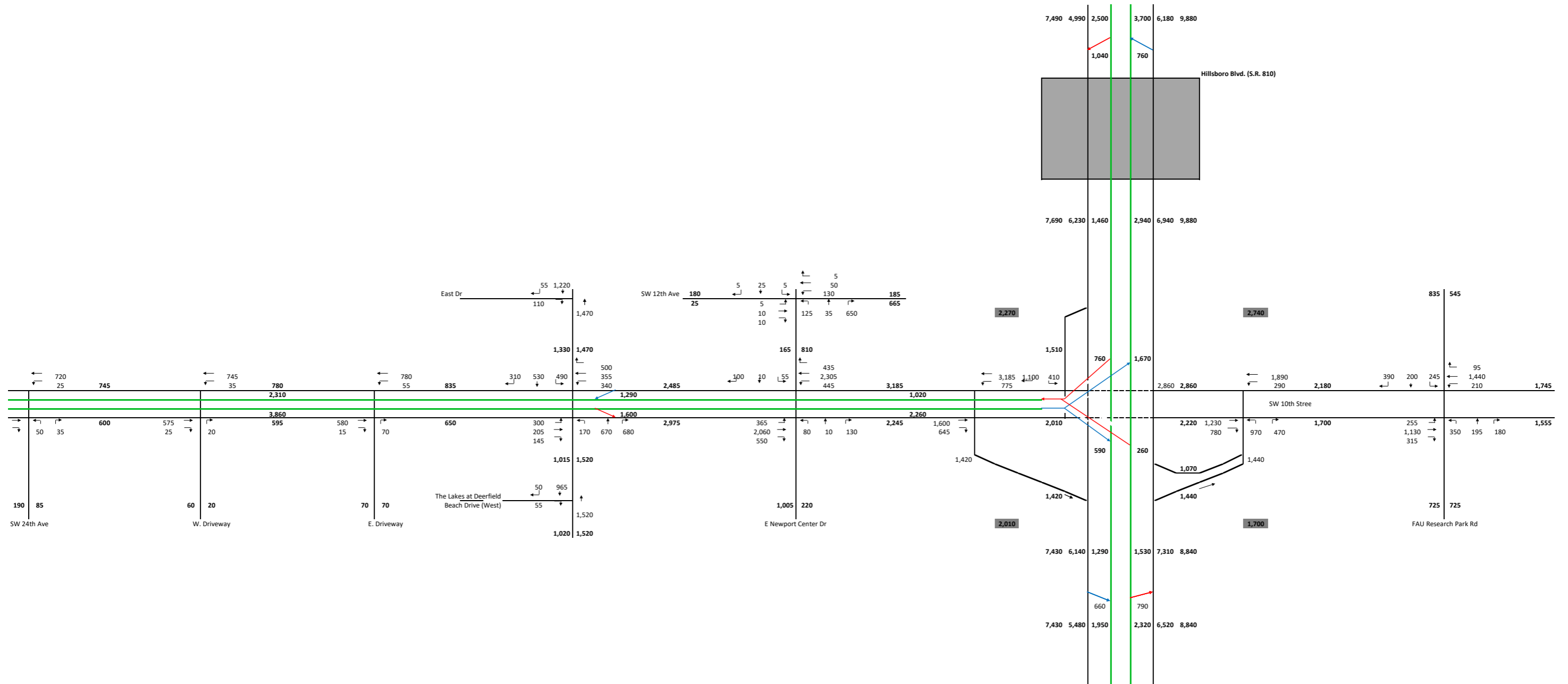
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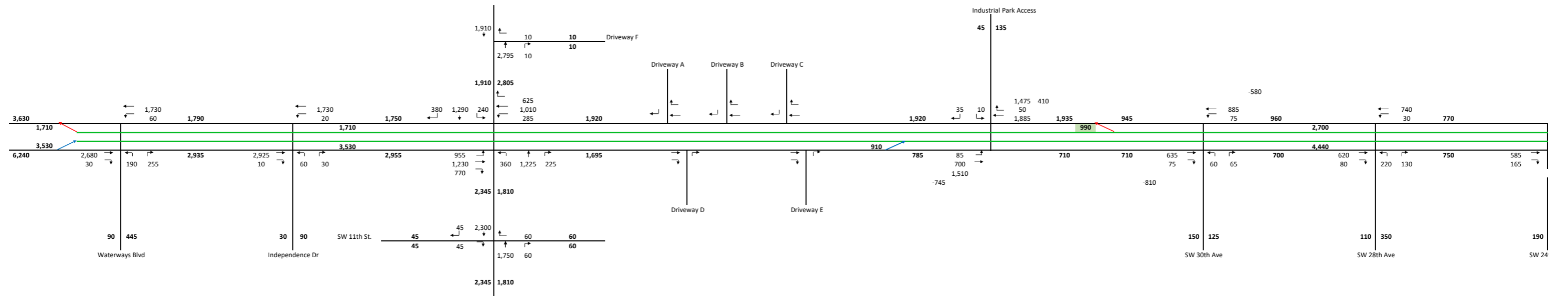
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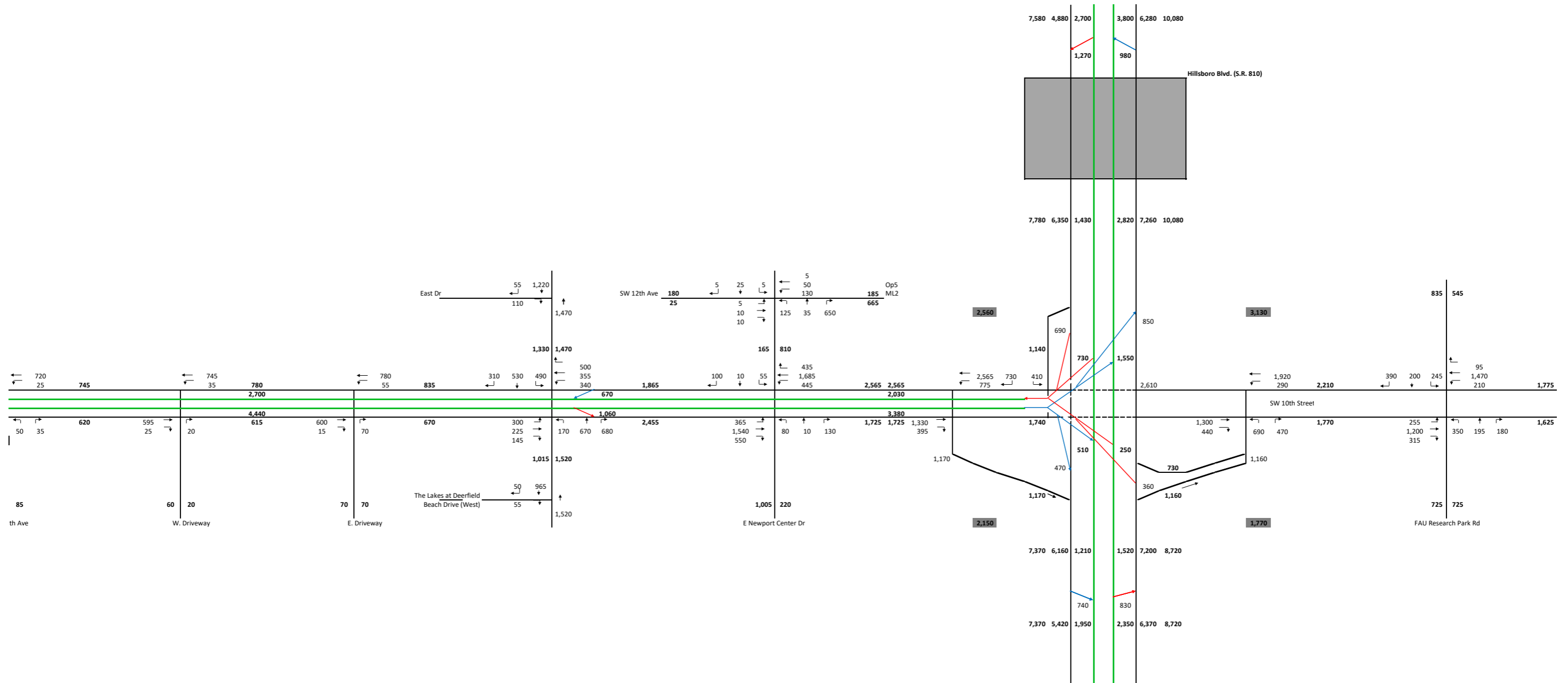
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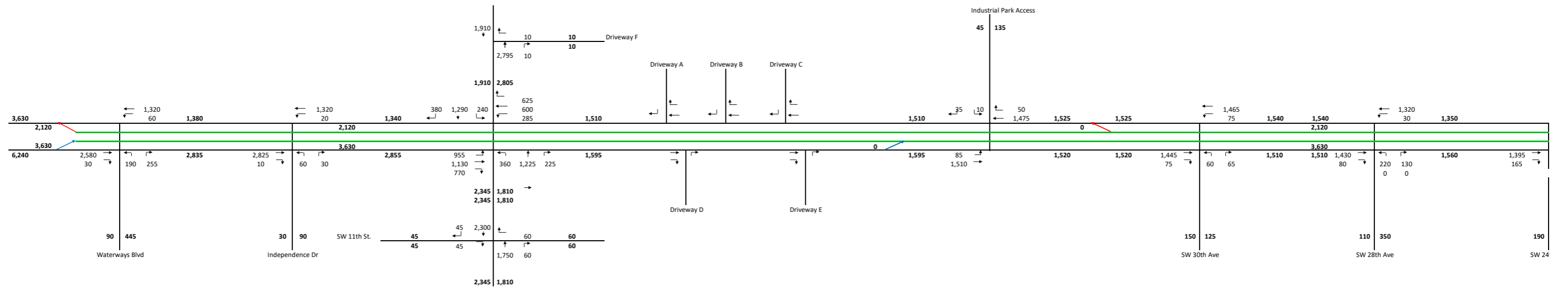
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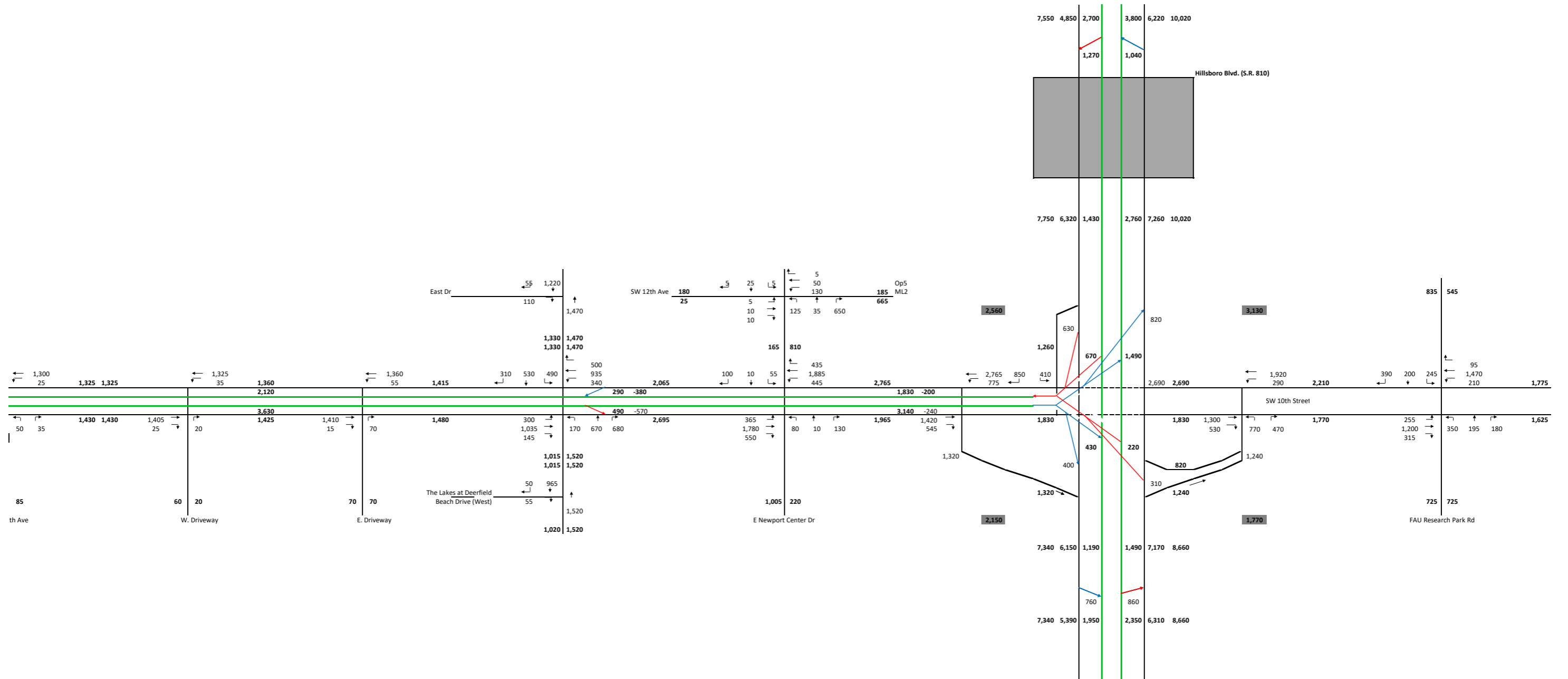
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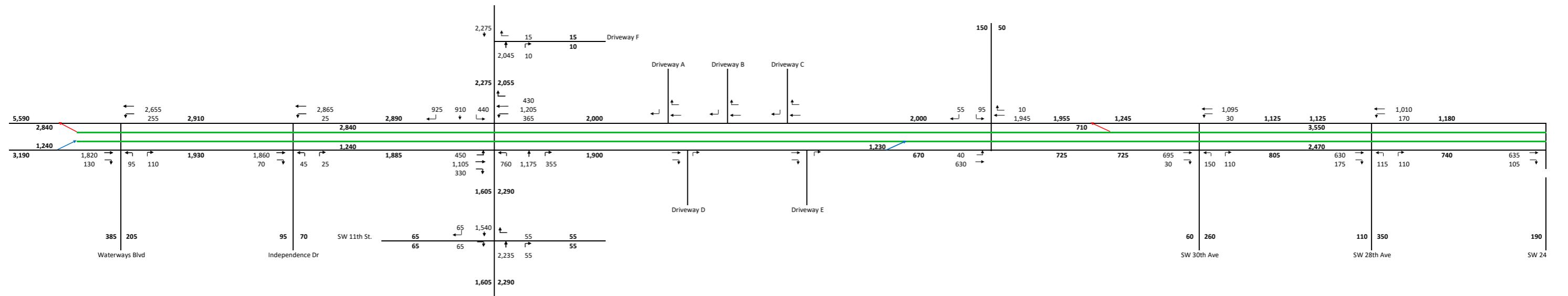
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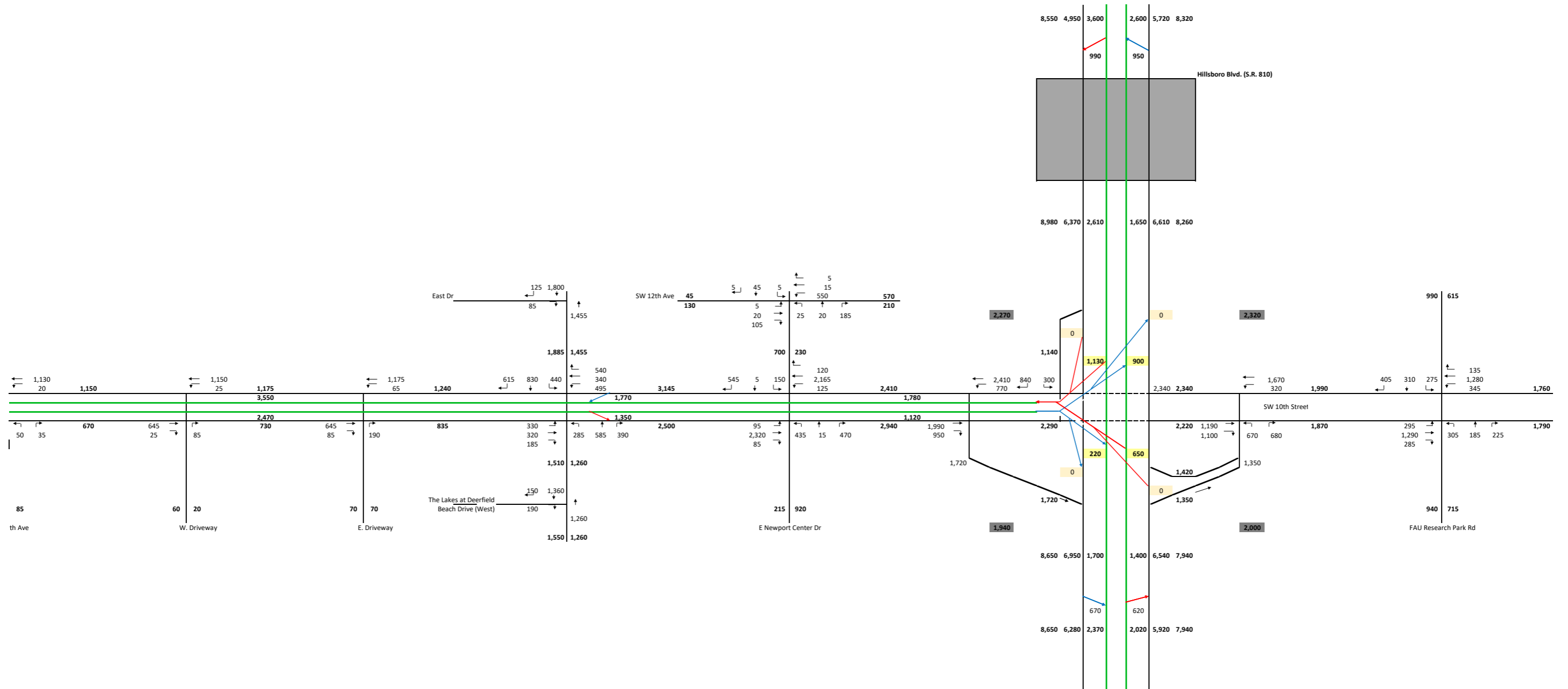
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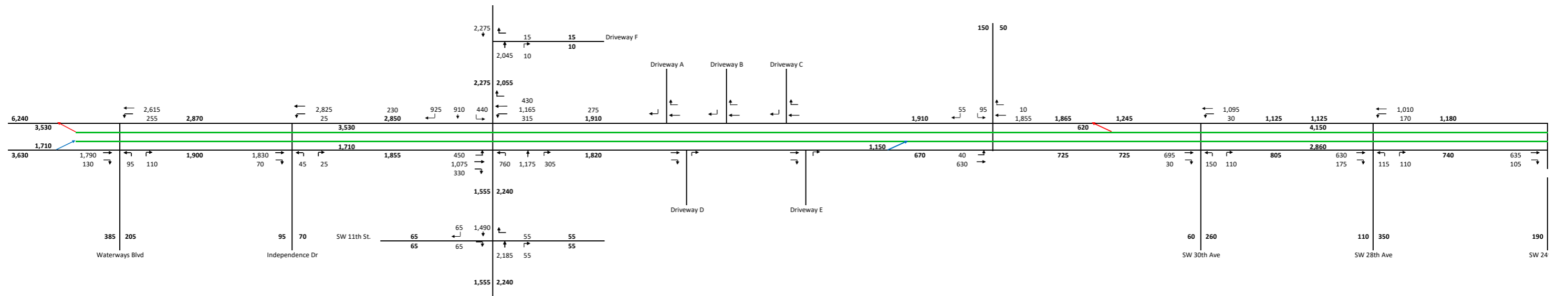
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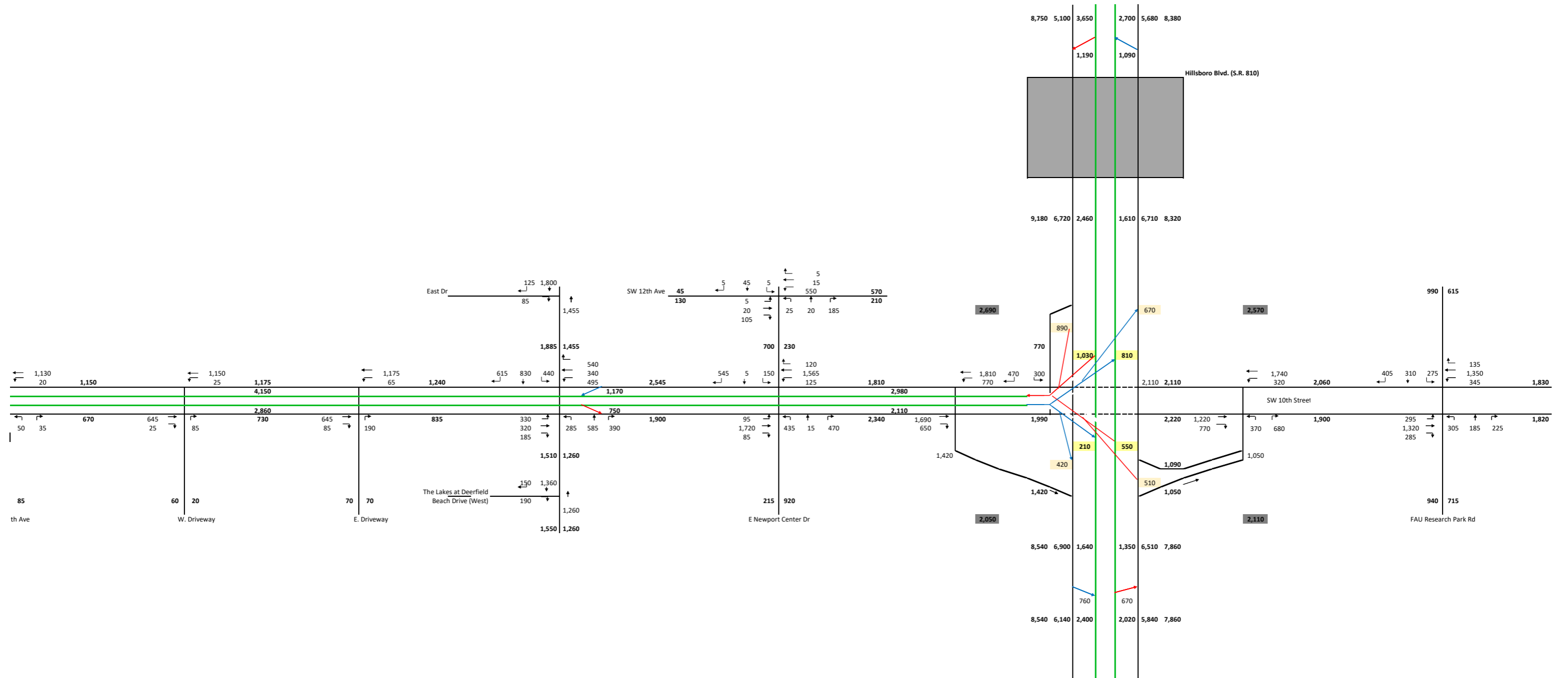
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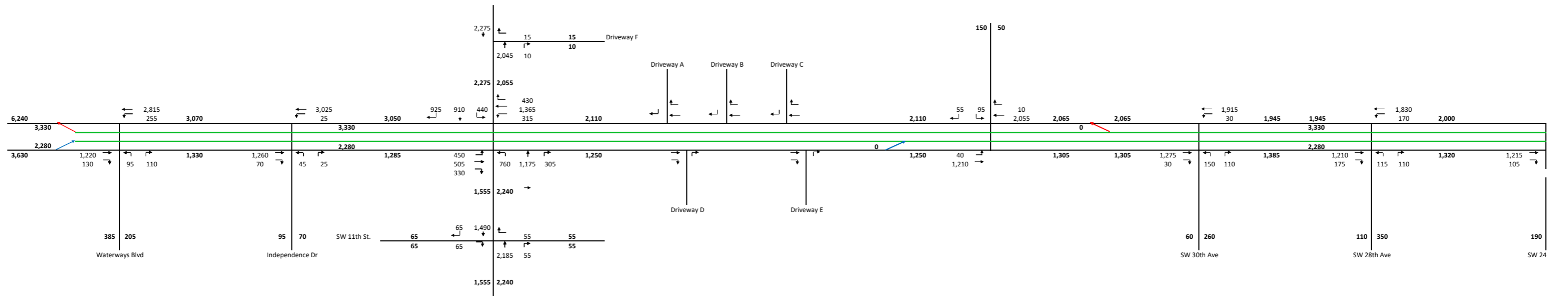
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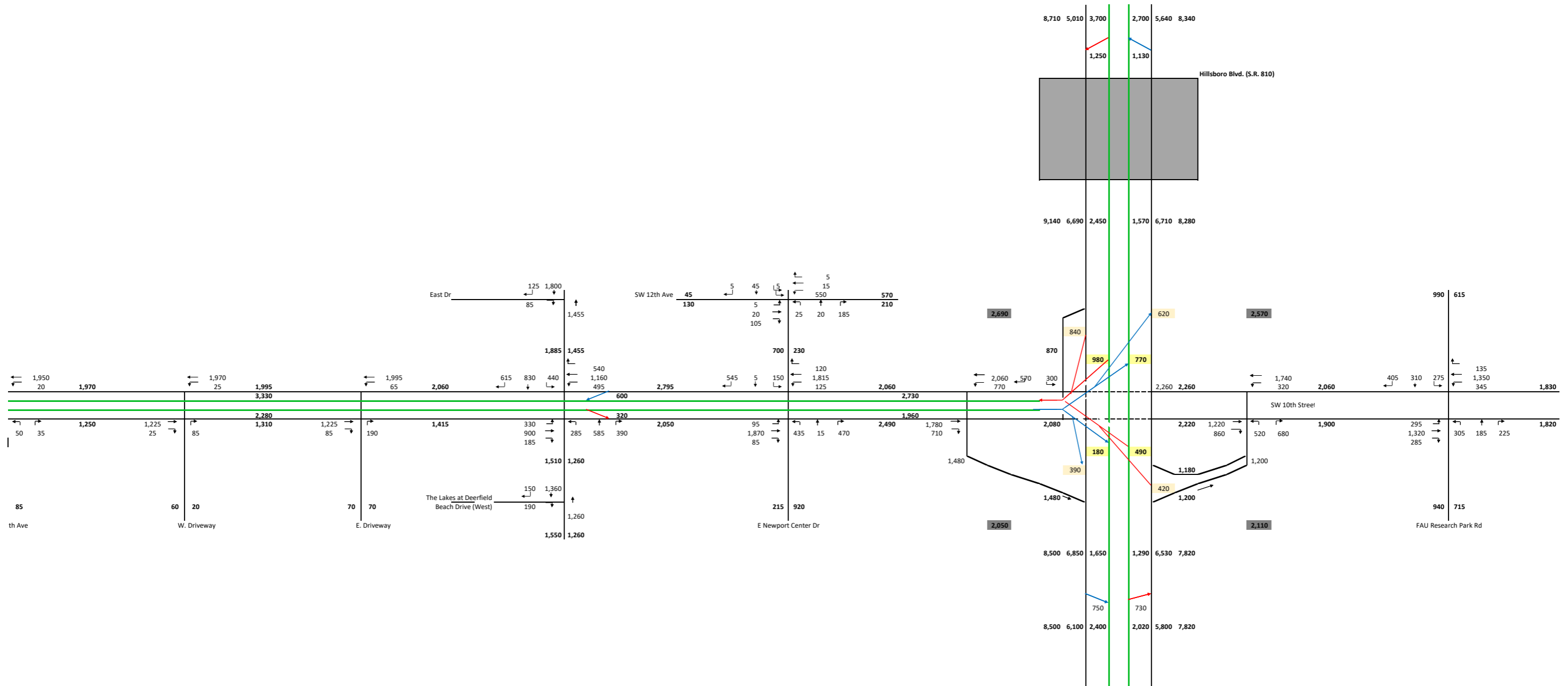
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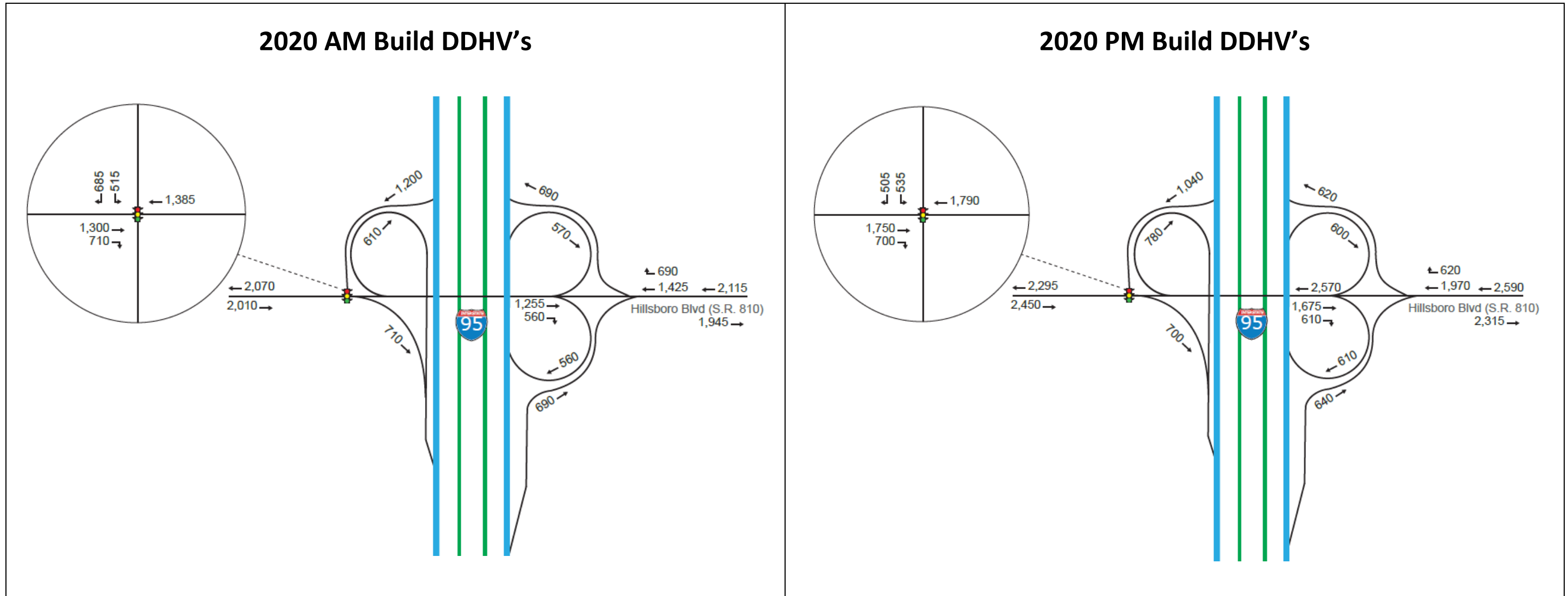
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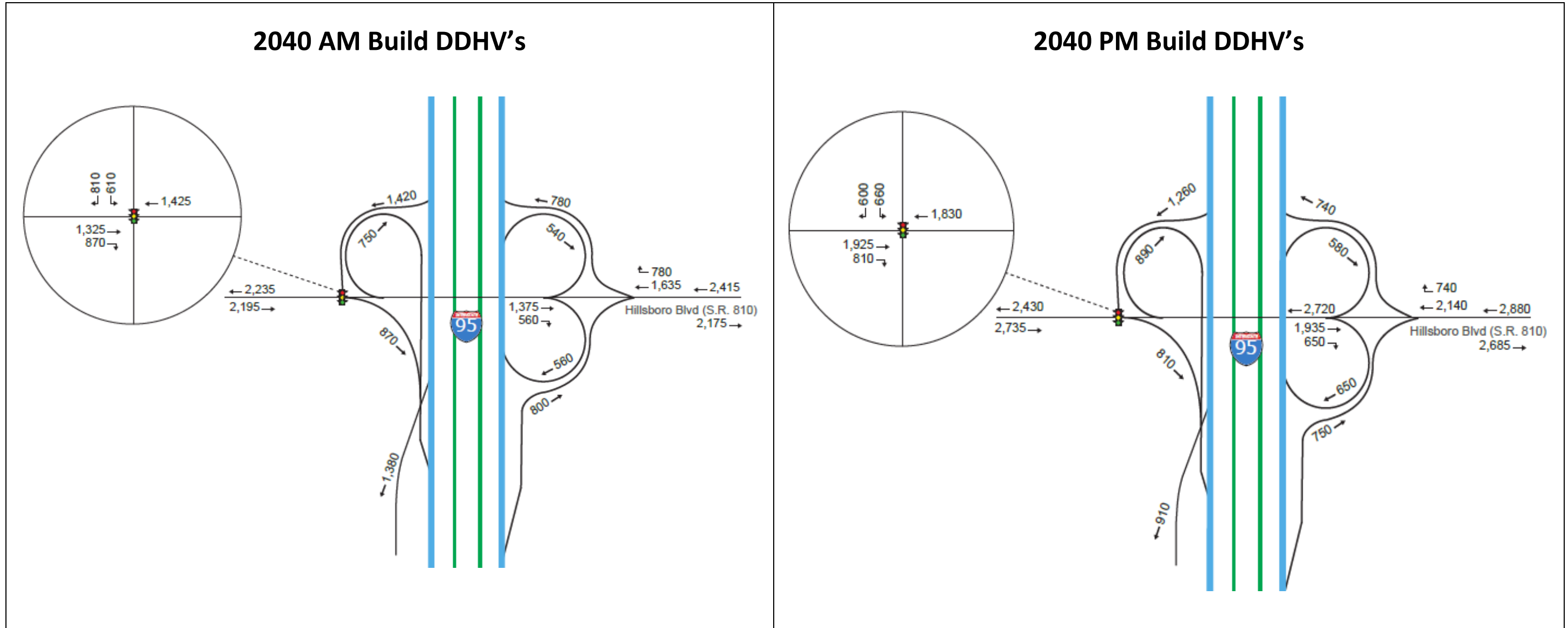
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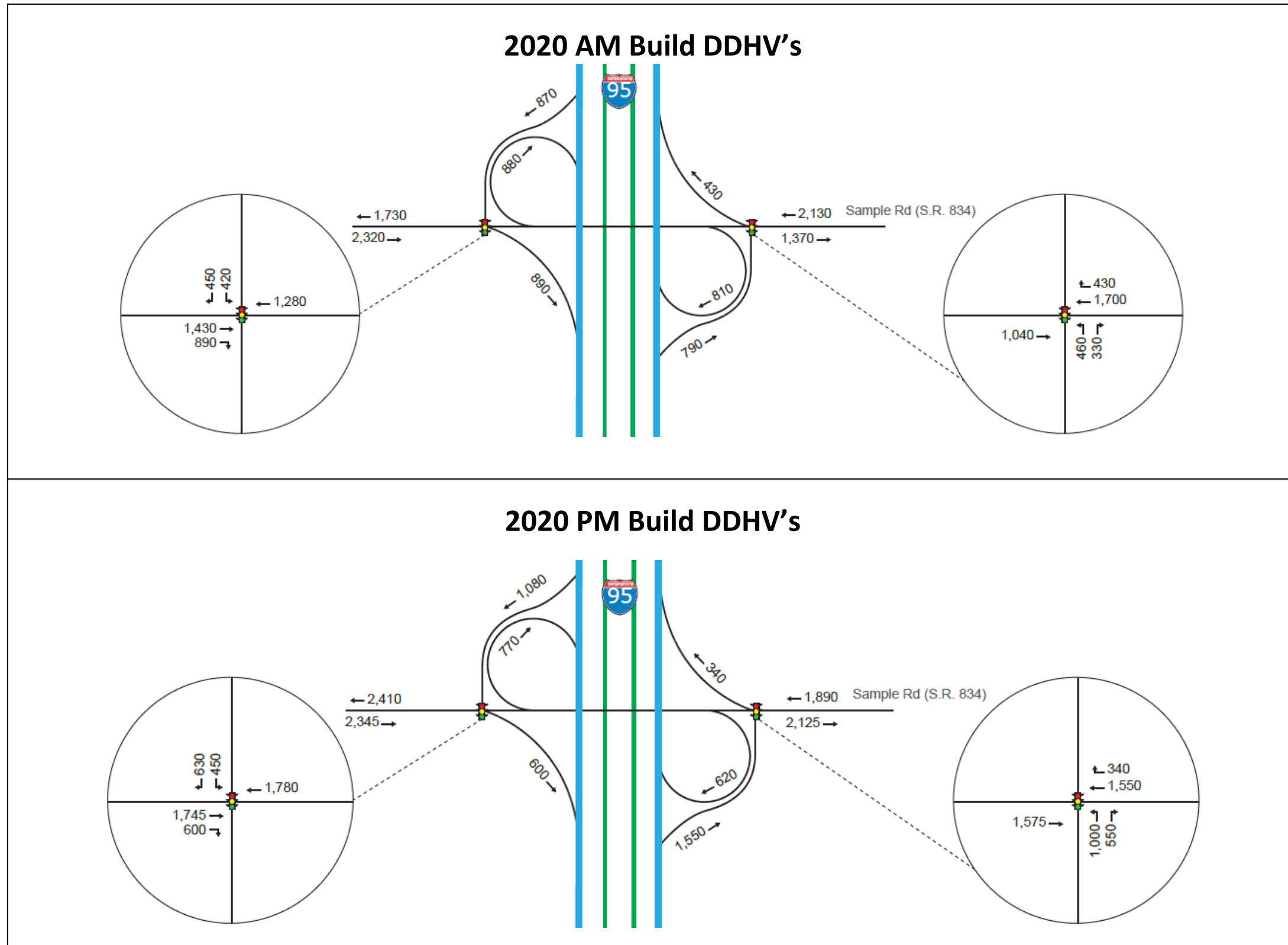
Base PD&E Concept, A2, and C2 – 2020 DDHV's Turns Hillsboro Blvd and I-95 (S.R. 9)



Base PD&E Concept, A2, and C2 – 2040 DDHV's Turns Hillsboro Blvd and I-95 (S.R. 9)

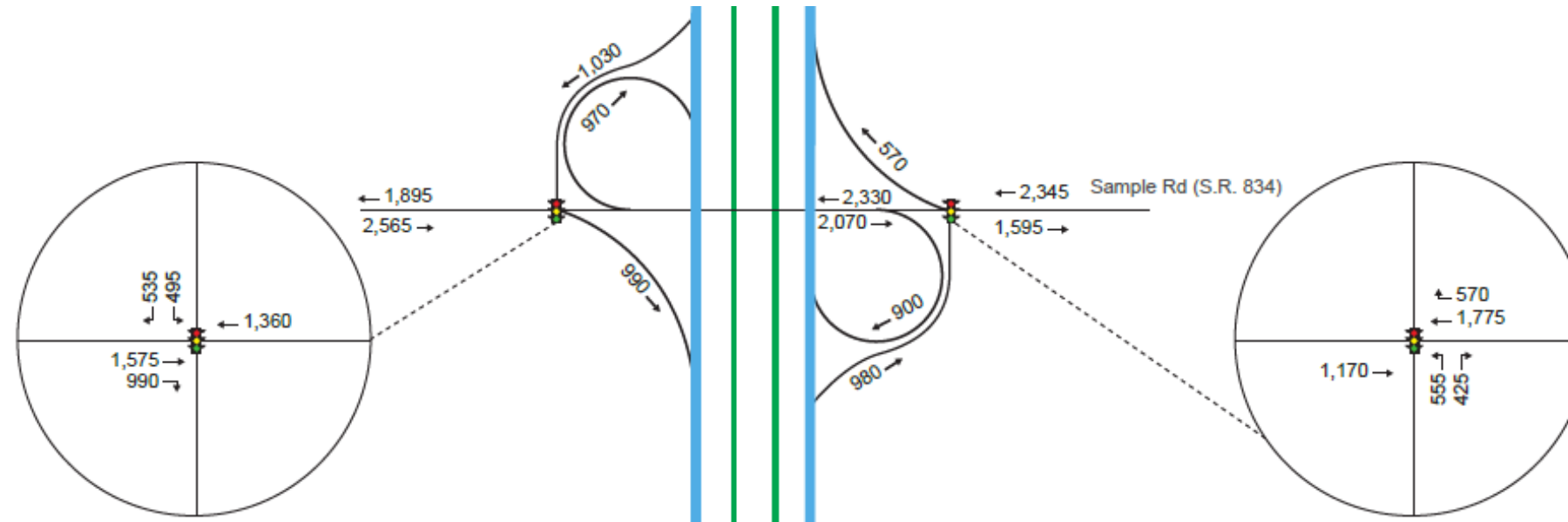


Base PD&E Concept, A2, and C2 – 2020 DDHV's Turns Sample Rd and I-95 (S.R. 9)

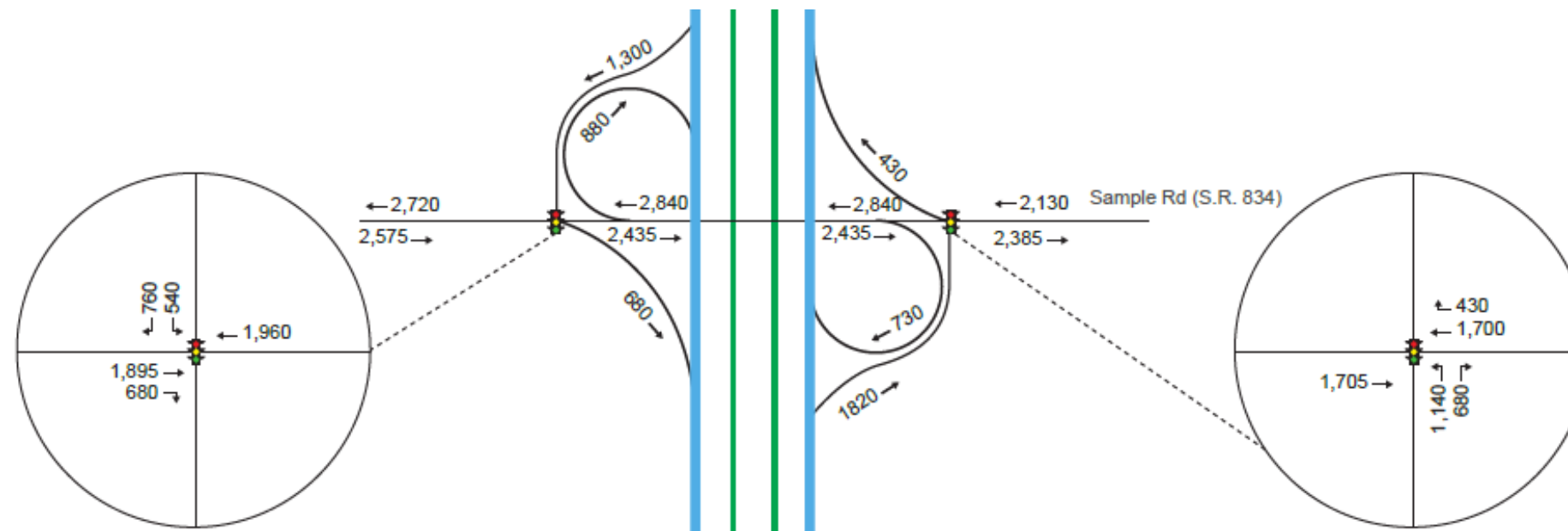


Base PD&E Concept, A2, and C2 – 2040 DDHV's Turns Sample Rd and I-95 (S.R. 9)

2040 AM Build DDHV's

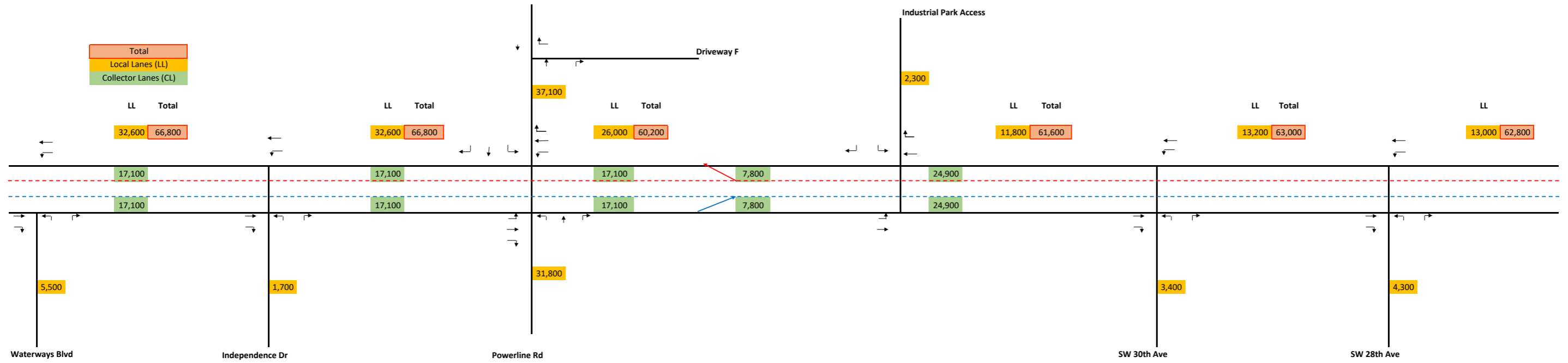


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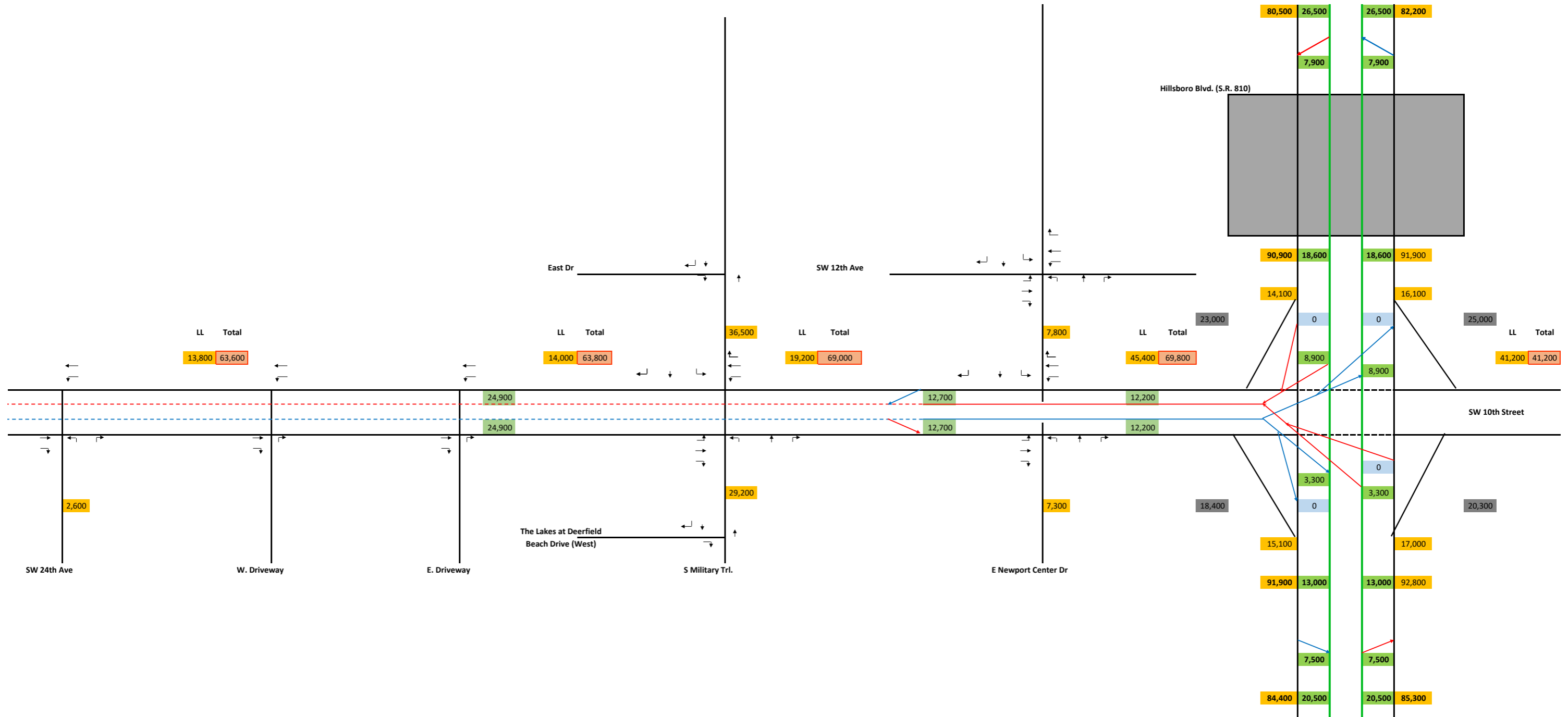


APPENDIX E

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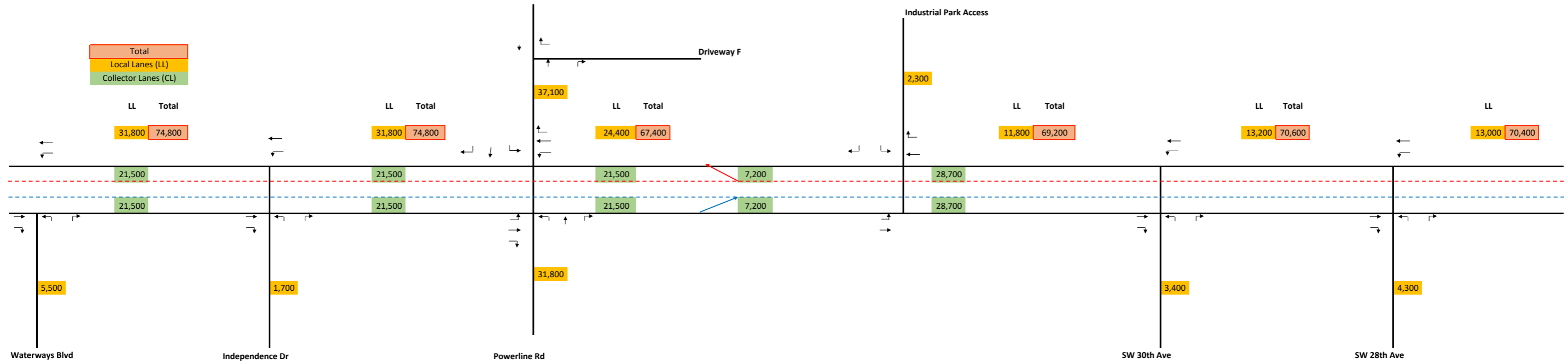


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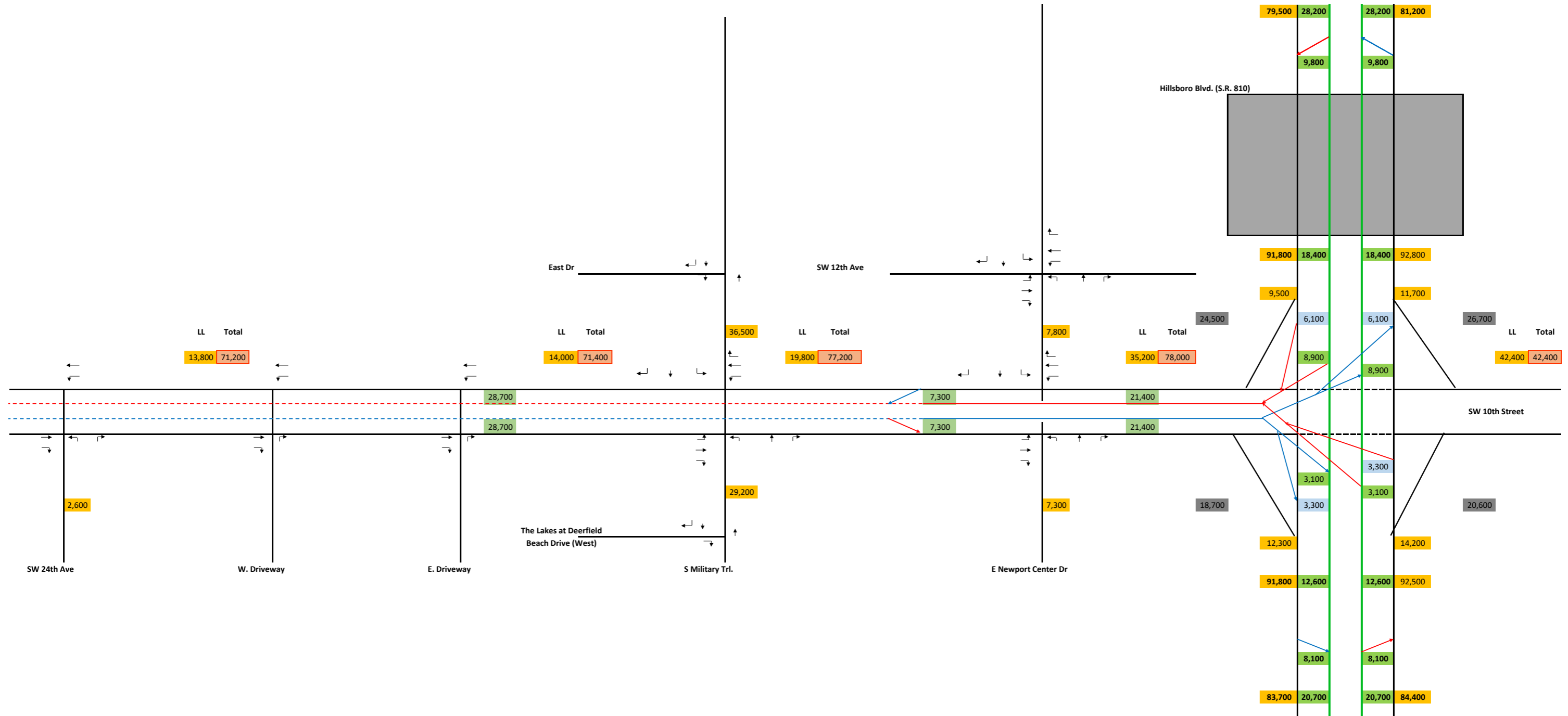


2020 AADT_Base PD&E

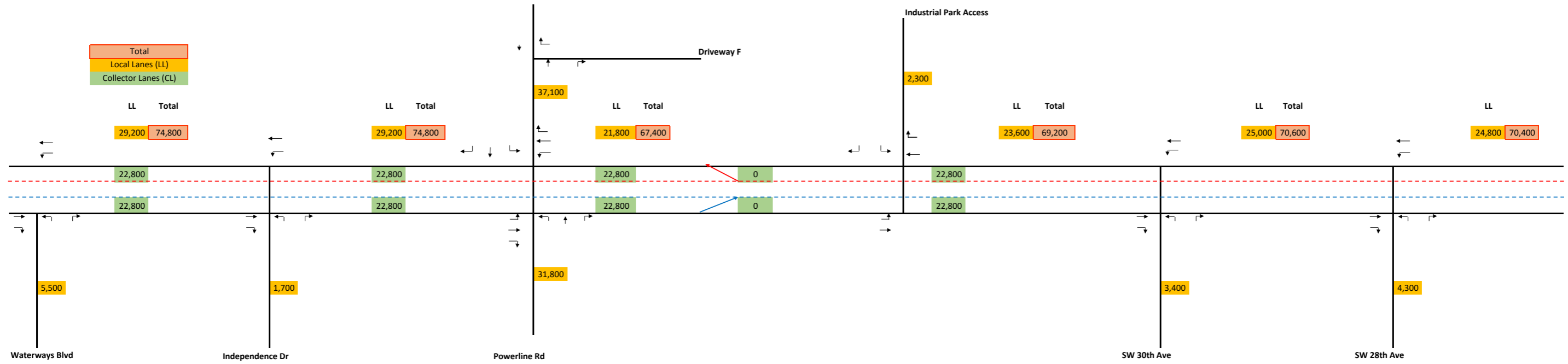
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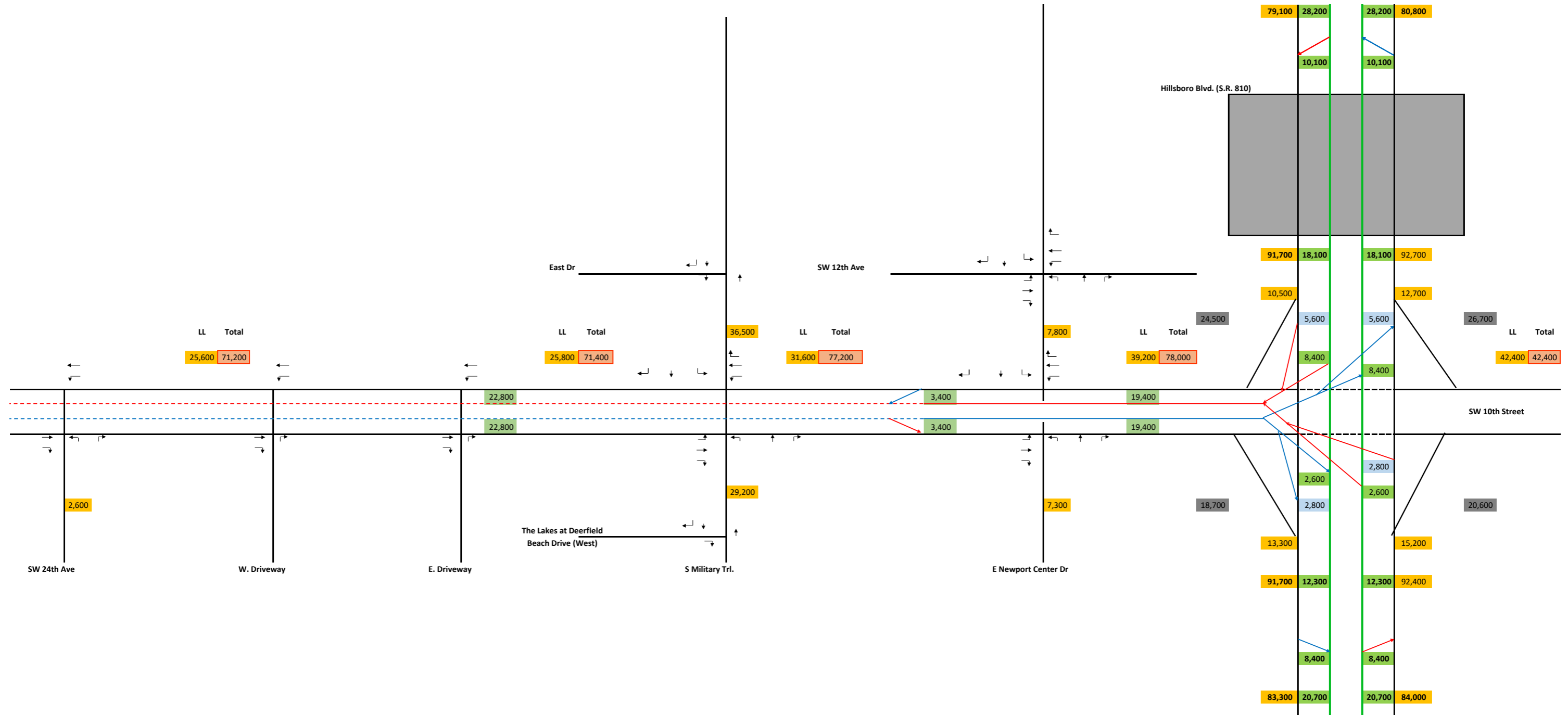
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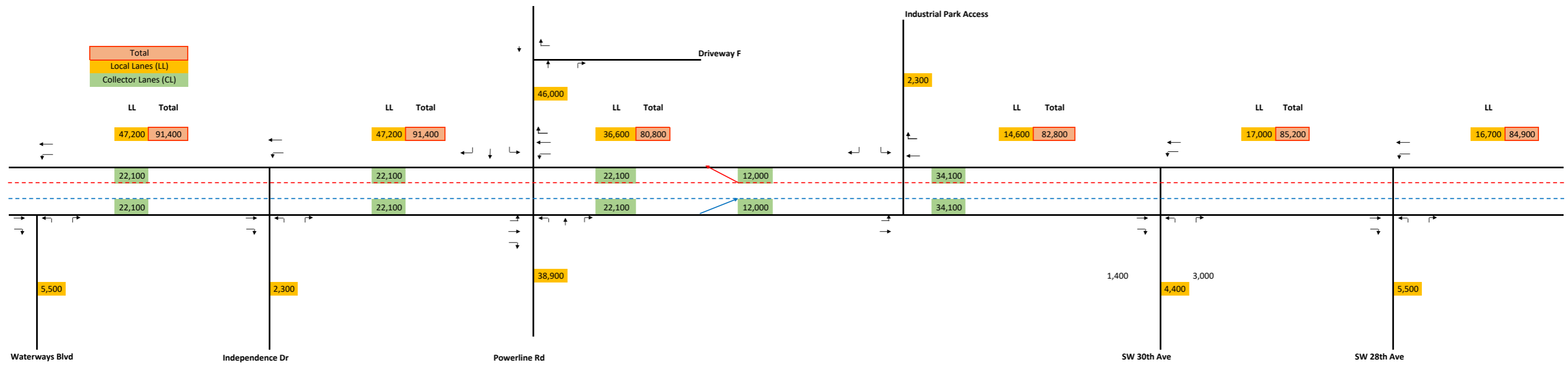
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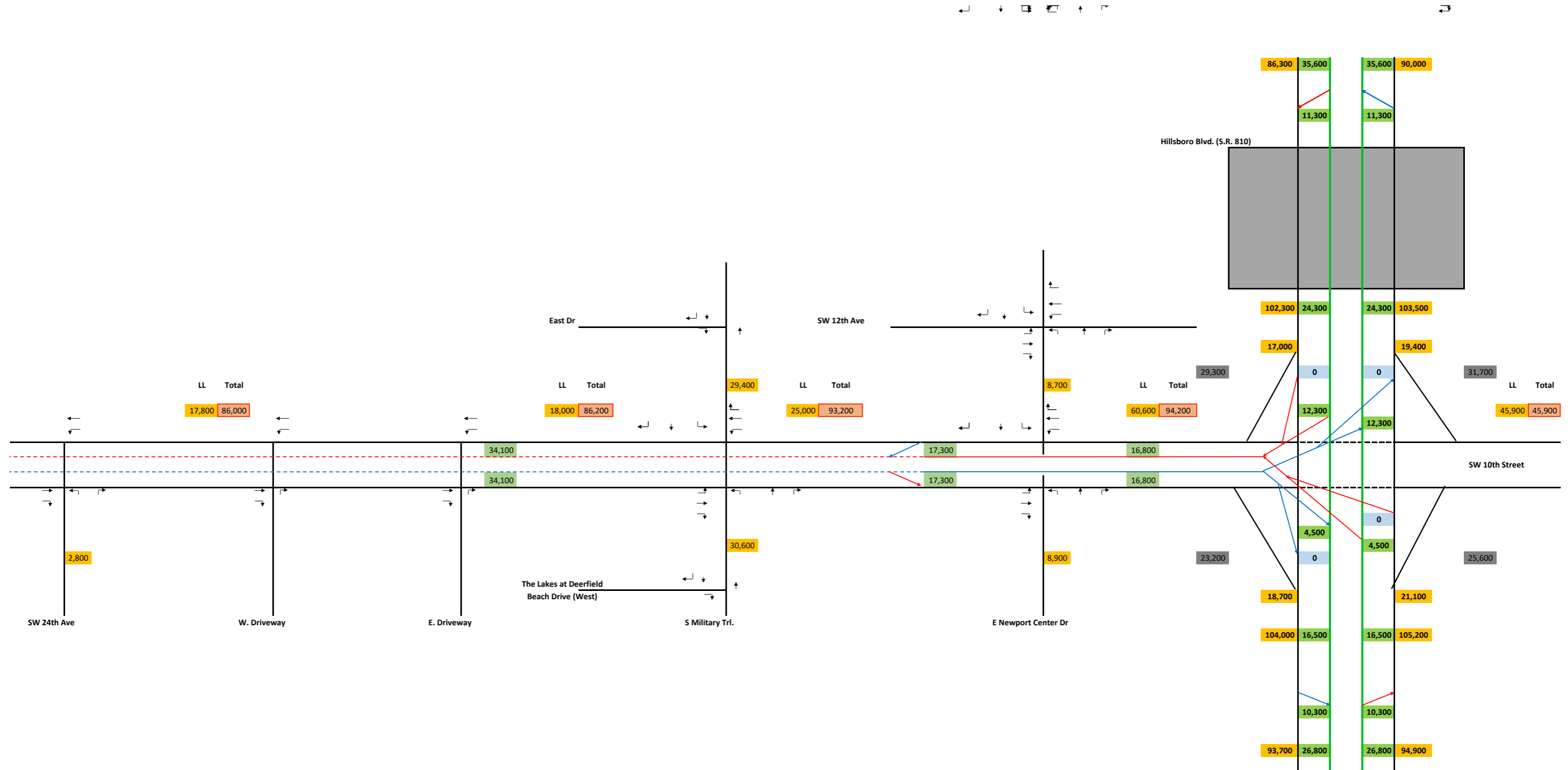
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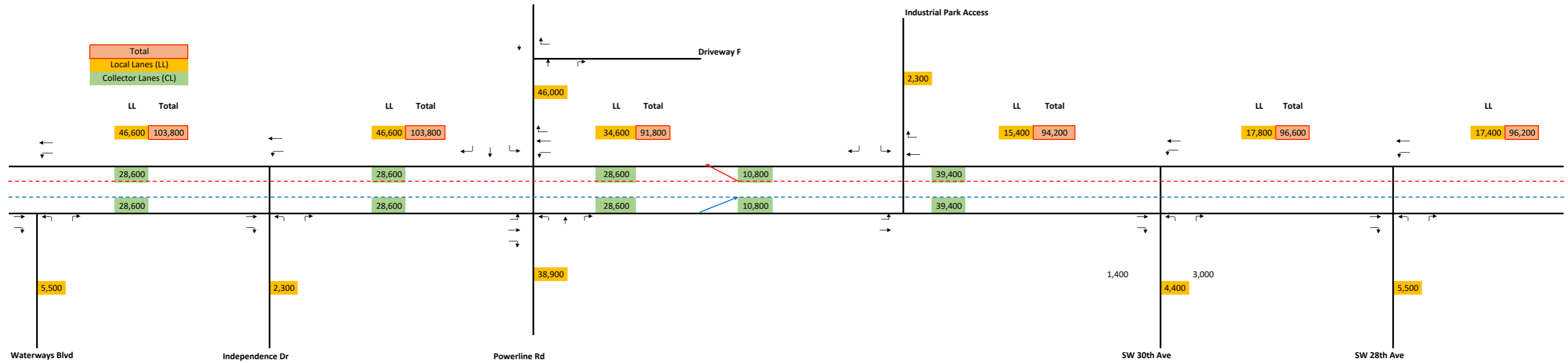


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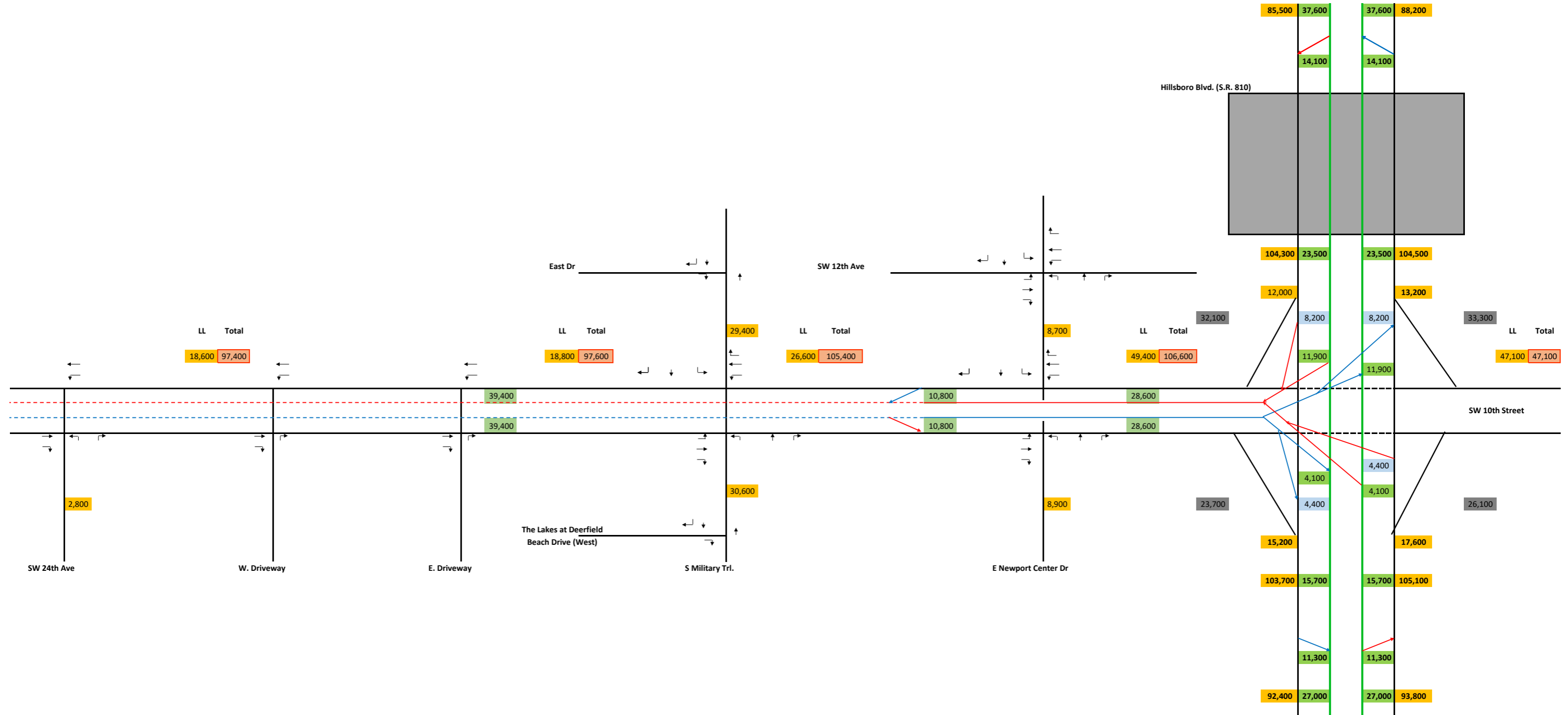


2040 AADT_Base PD&E

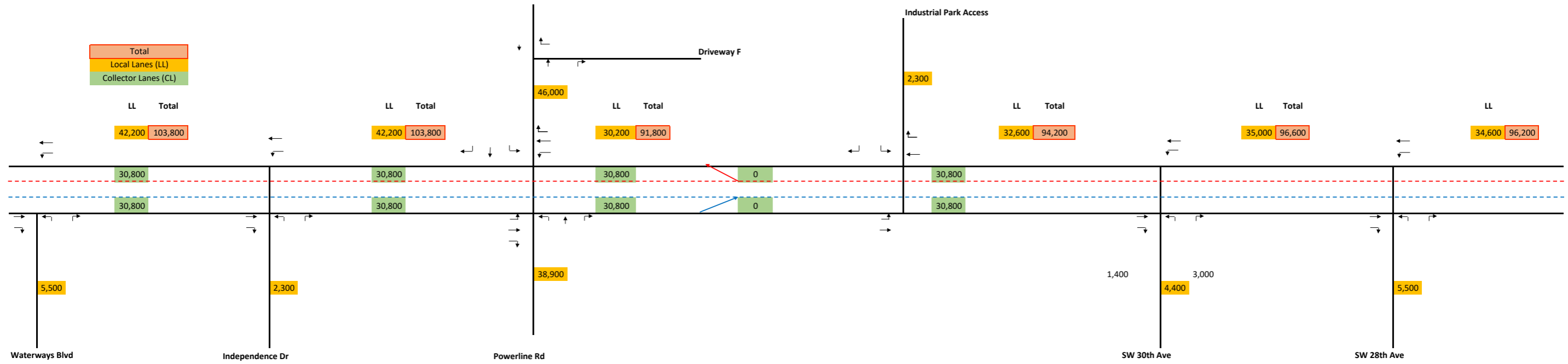
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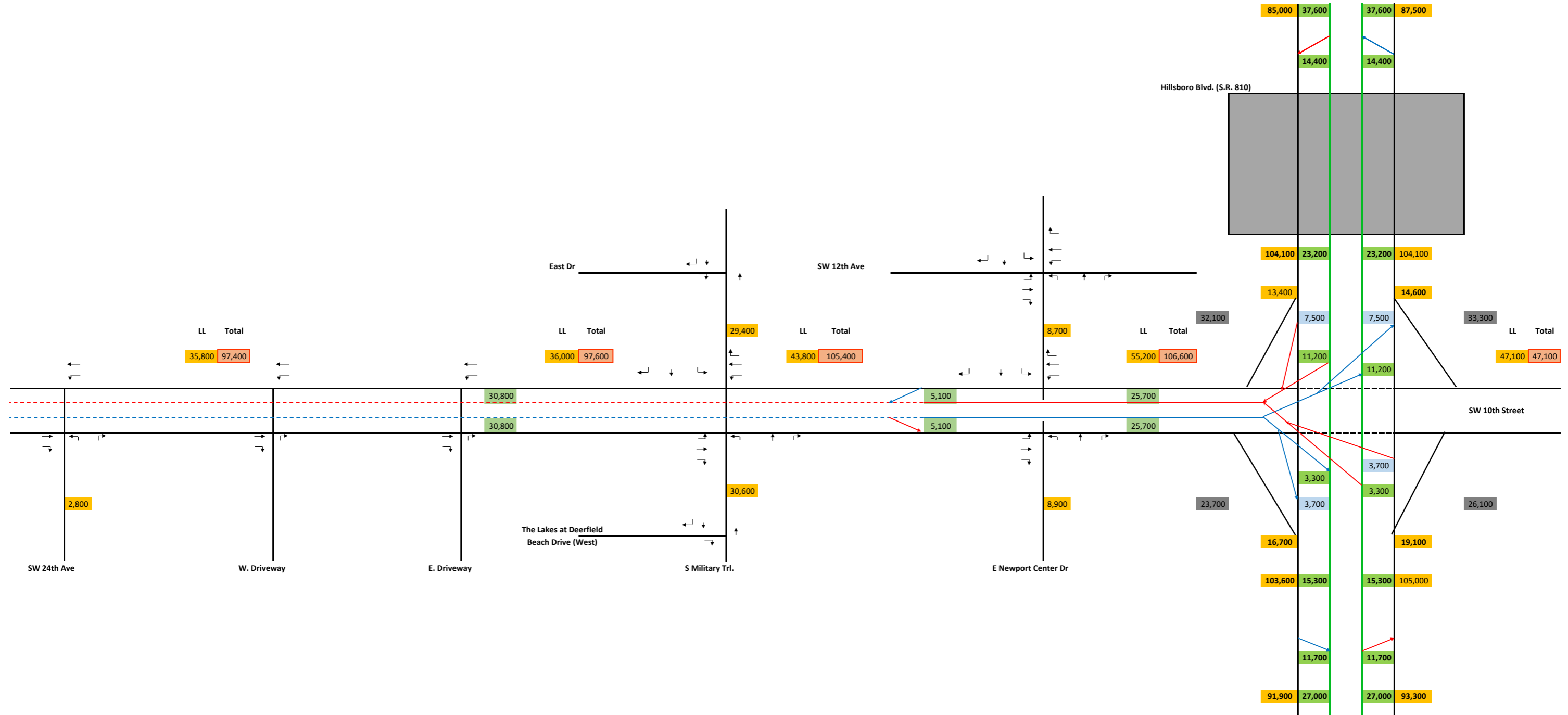
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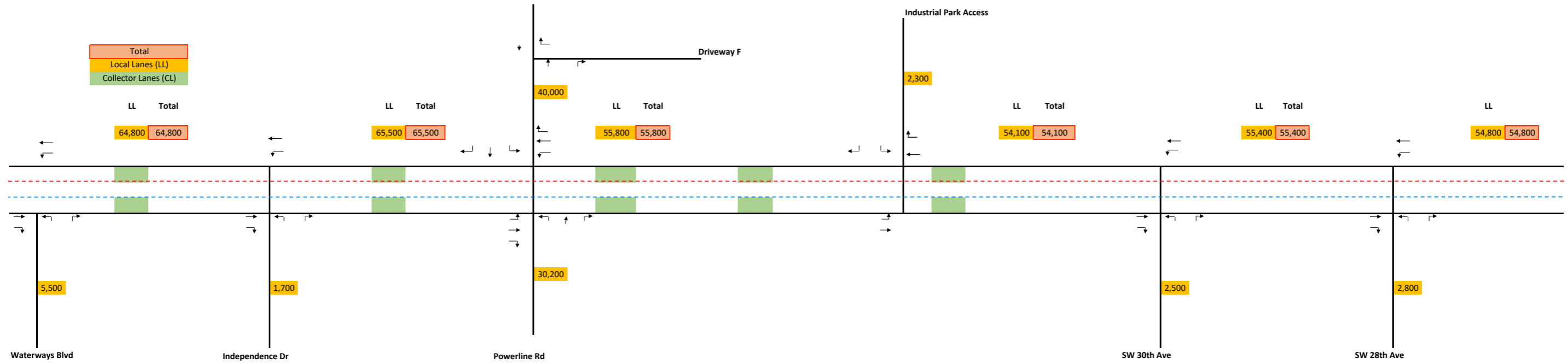


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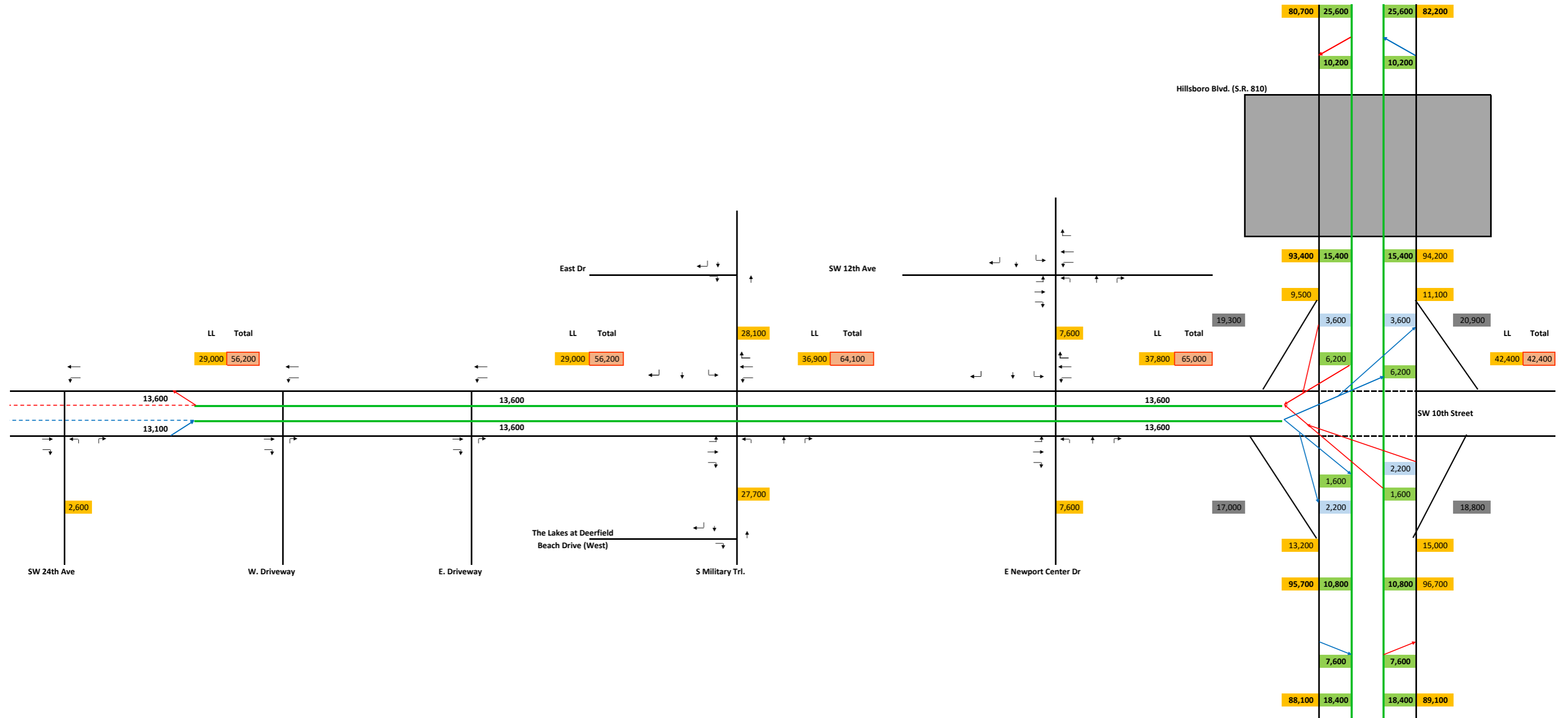


APPENDIX F

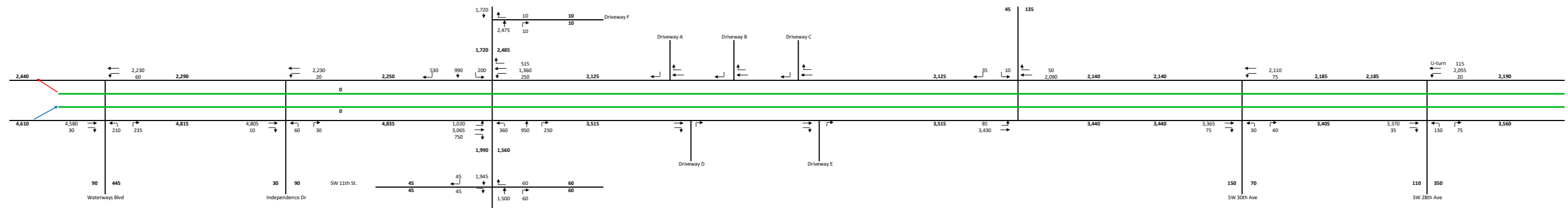
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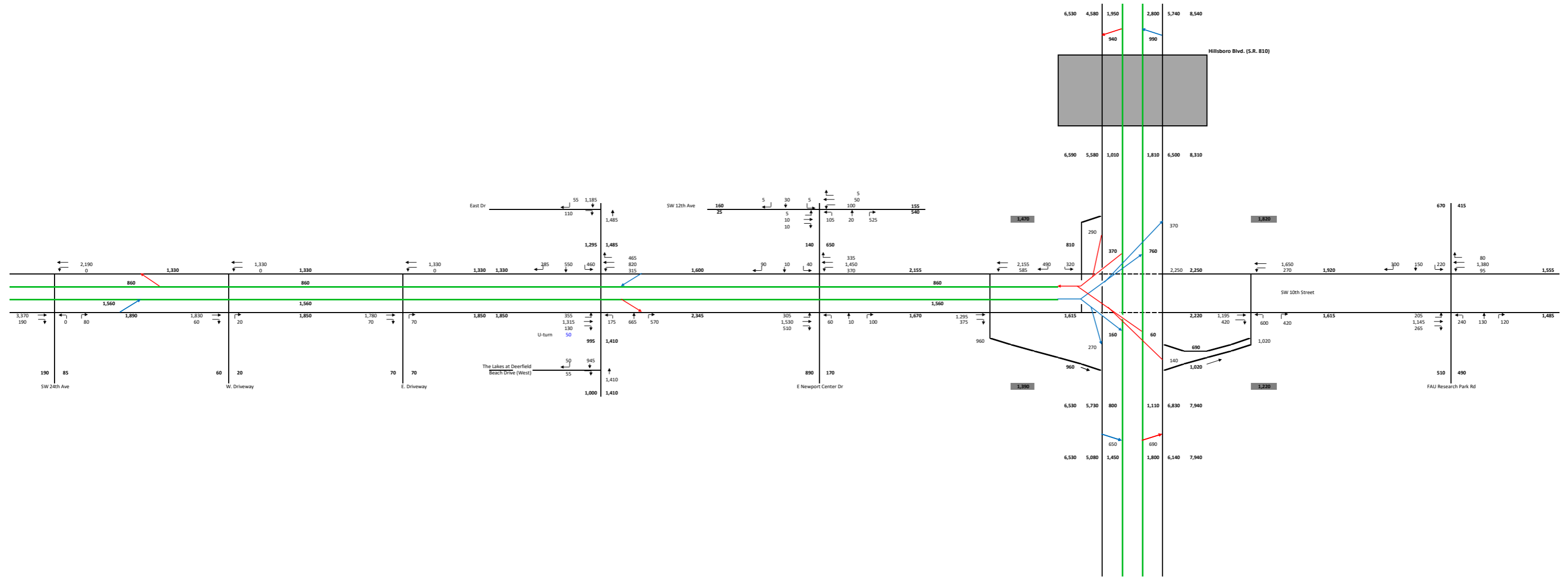
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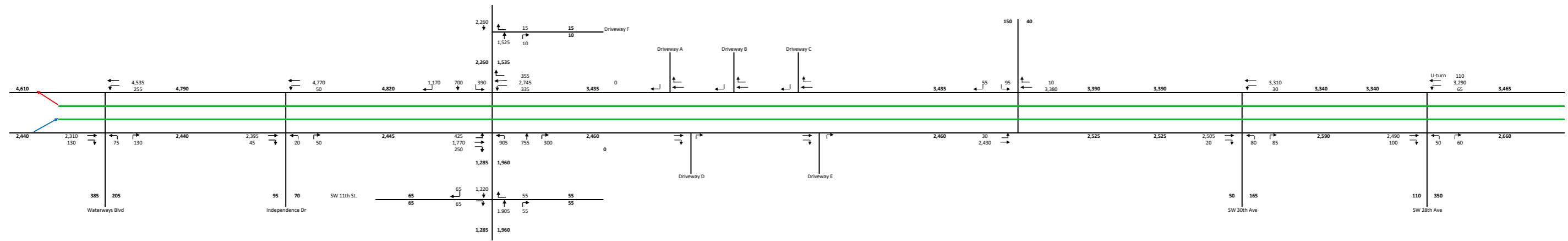
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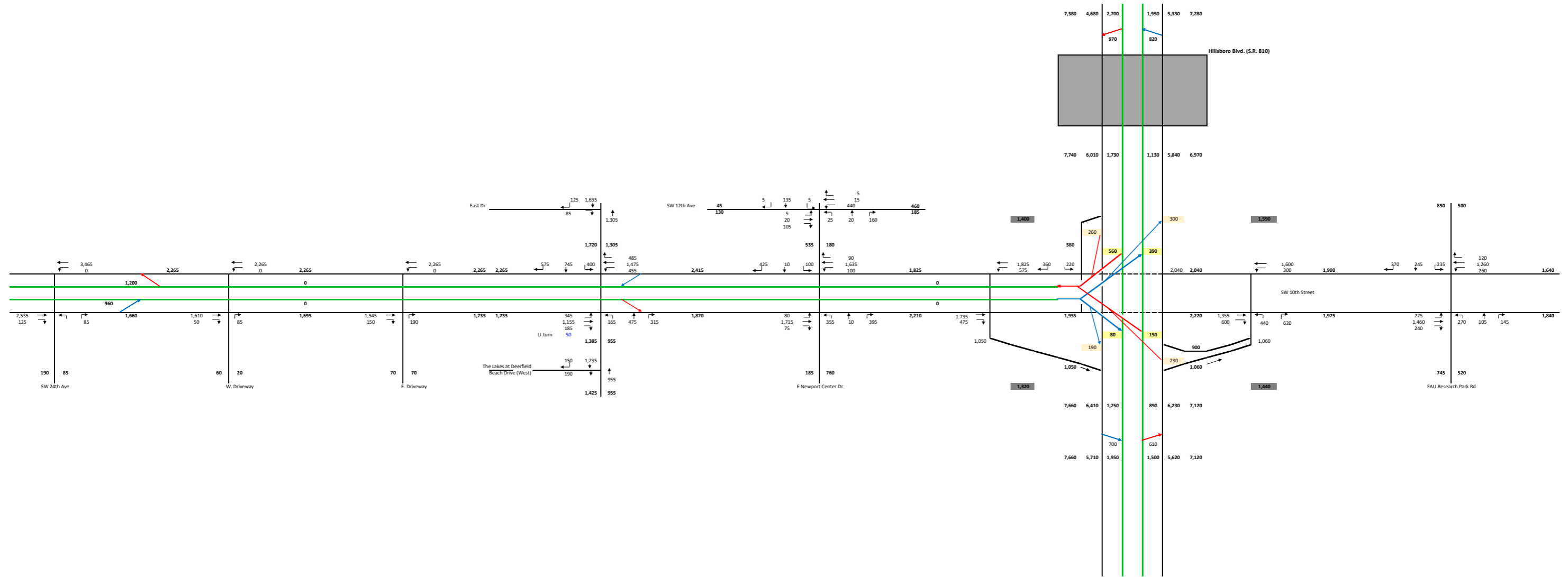
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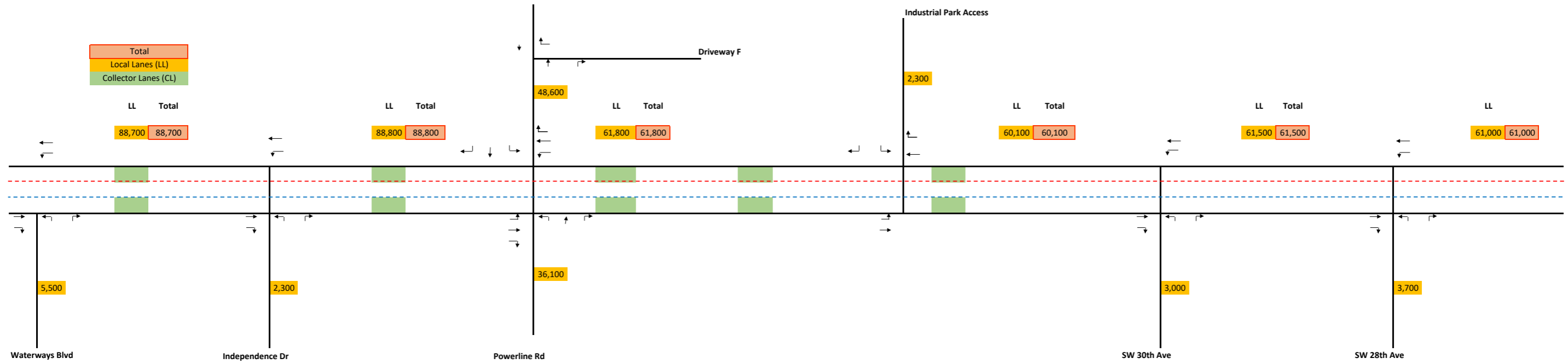
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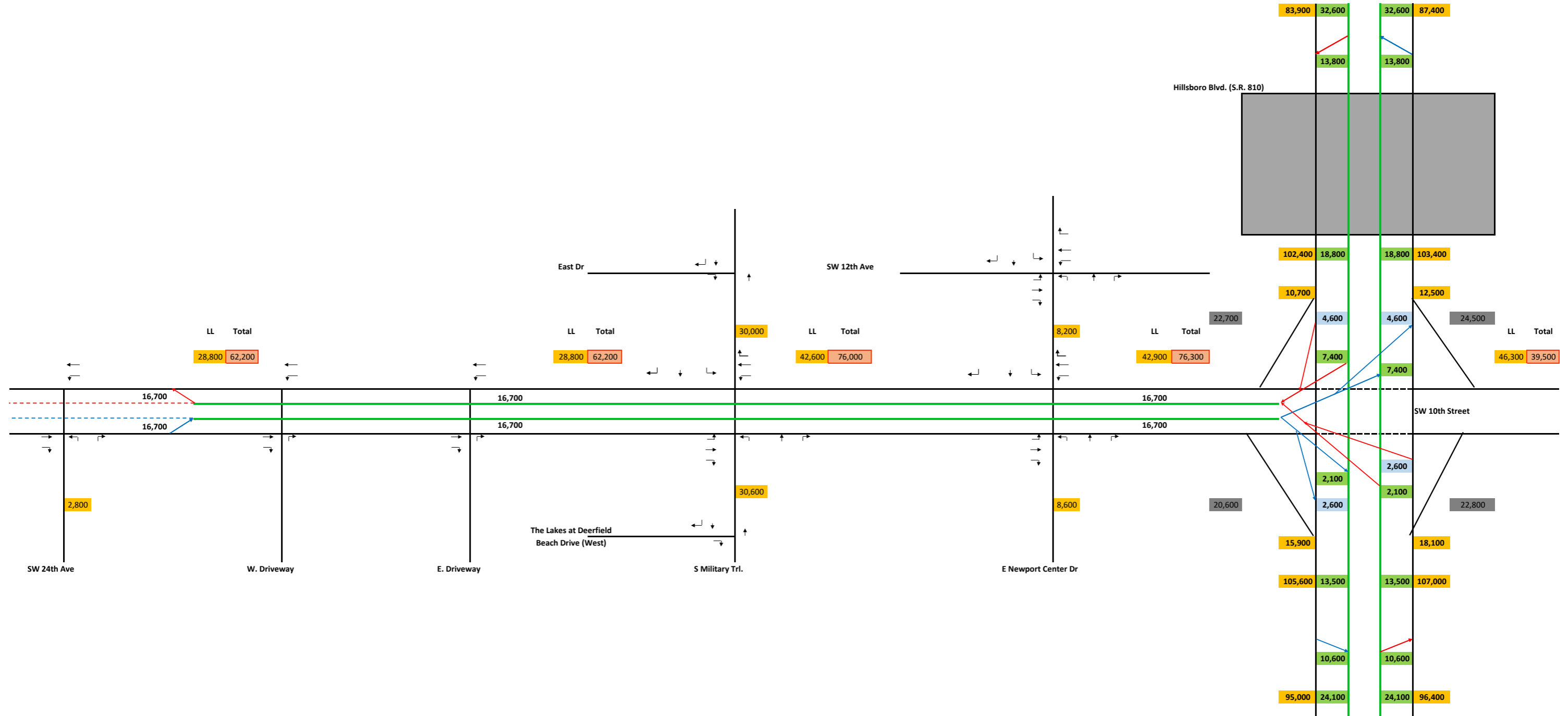
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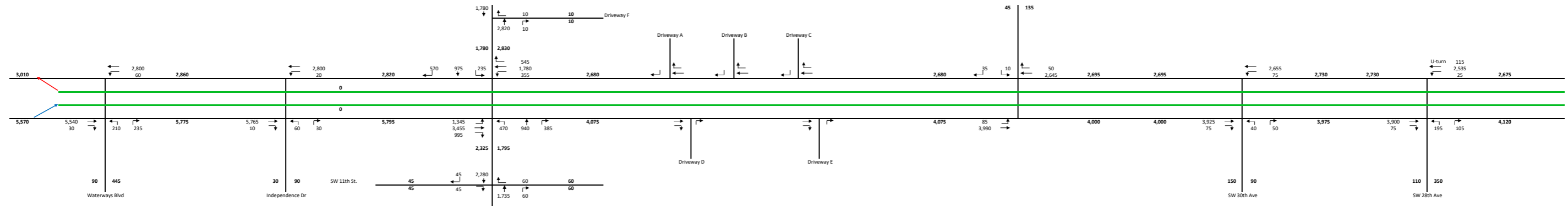
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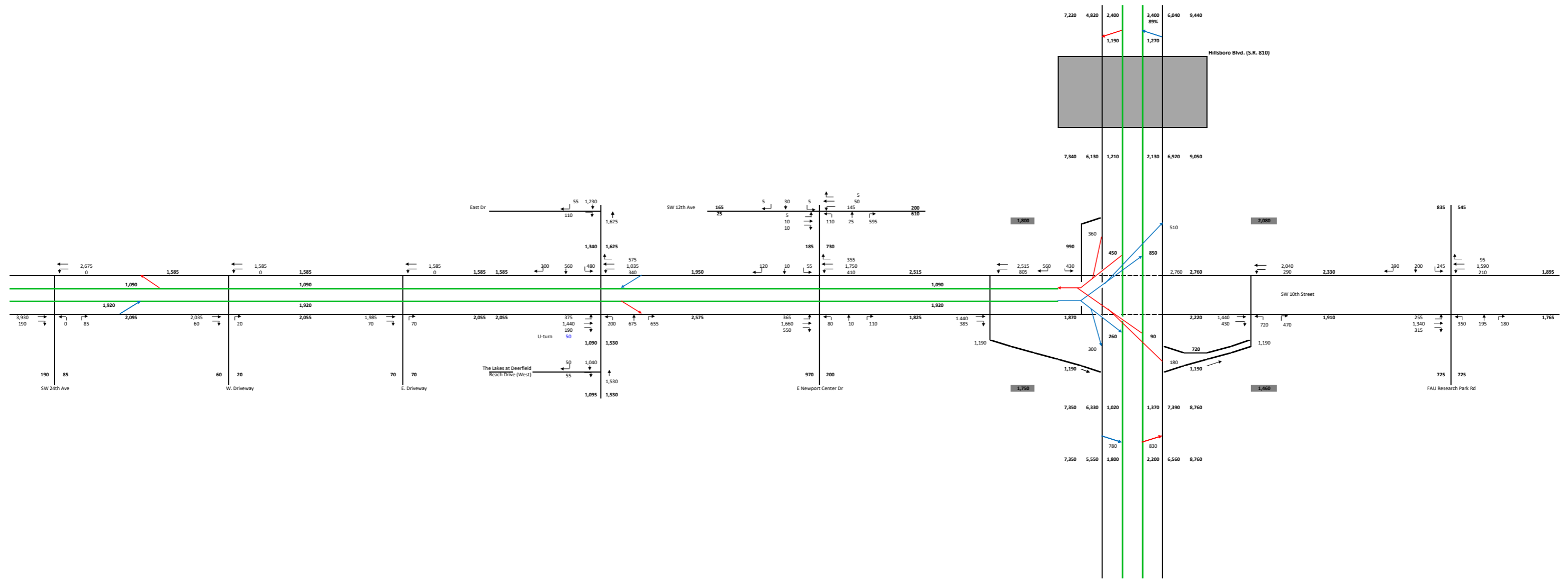
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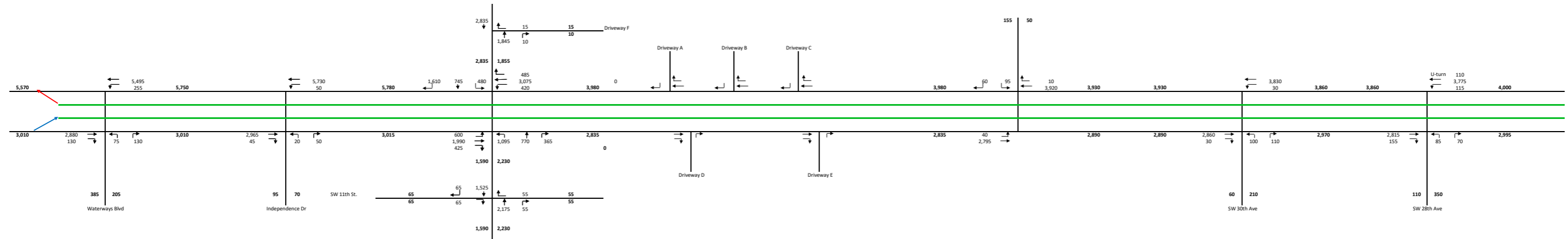
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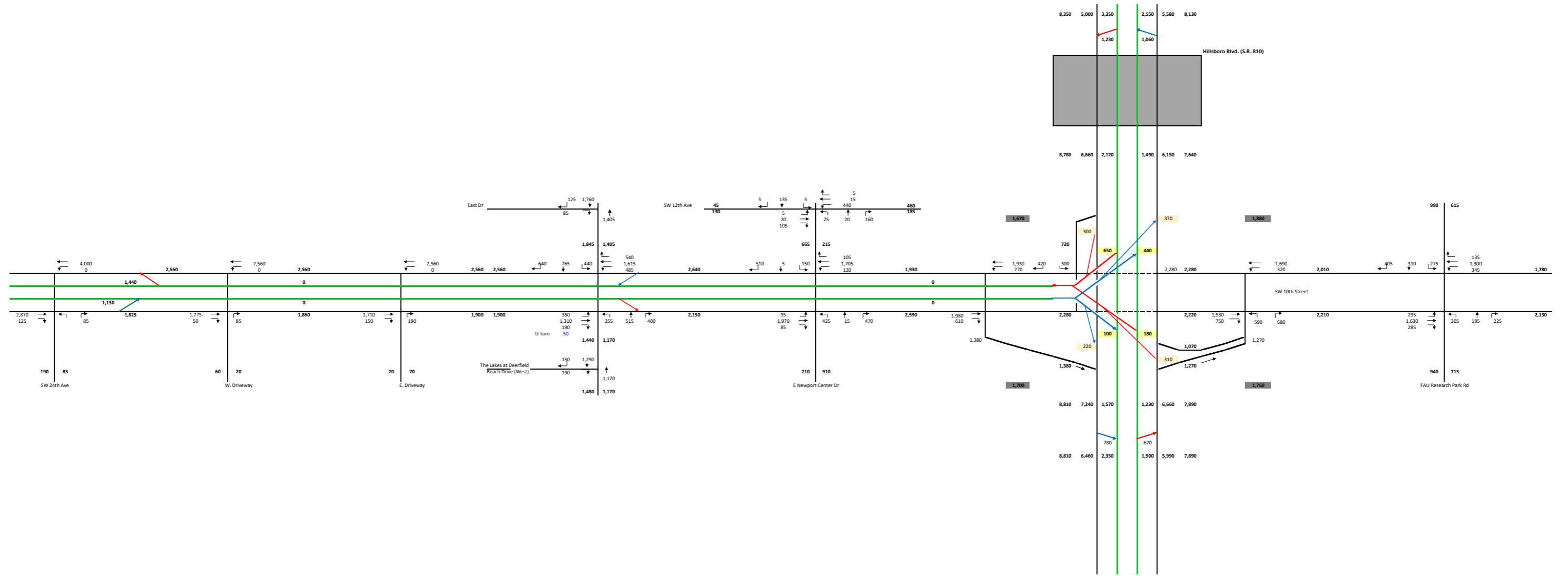
Appendix F



Appendix F



Appendix F



SW 10th Street Connector Project Eastbound Weave Operations at the Connector Lane Egress Technical Memo

Date: Monday, March 08, 2021

Project: SW 10th Street and I-95 Connector Design Consultant (CDC)

Prepared by: Megan McGinley and Will Suero (HDR)

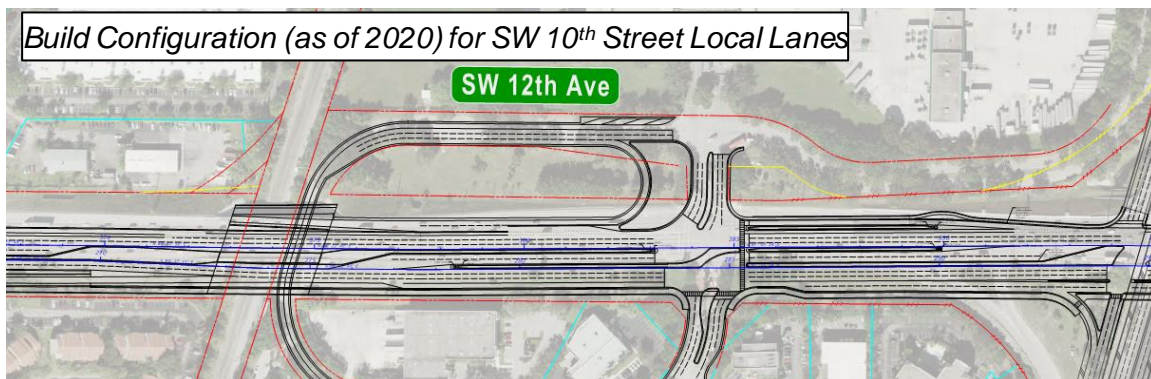
Subject: SW 10th Street Eastbound Weave Operations from the Connector Lane Egress, West of Newport Center Drive, to the Newport Center Drive and I-95 Intersections

Attachments: (2021) Traffic Analysis Presentation for Newport Center Drive Intersection Analysis
(2018) Alternatives Analysis Memorandum for SW 10th Street at I-95

The intersection of SW 10th Street at Newport Center Drive was reevaluated to determine whether a full intersection opening could provide acceptable operations and be implemented over a Restricted Crossing U-Turn (RCUT) intersection. A Vissim traffic analysis was conducted by the CDC Team in January 2021 where network-level segment speeds and intersection-level movement performance measures were compared for the Design Year of 2040. Prior to this Vissim analysis, the Intersection Control Evaluation (ICE) process and a Synchro analysis was conducted to screen potential intersection configurations. While the analyses (**attached**) showed that the full intersection configuration could still accommodate the 2040 traffic volumes, **the purpose of this memo is to document:**

(1) whether the weaving movement from the connector lane egress, west of Newport Center Drive, to the eastbound left-turn at SW 12th Ave/Newport Center Drive as well as the I-95 ramp terminal intersections is feasible based on traffic demand and design guidance, and

(2) how the full intersection opening affects traffic operations for the overall eastbound weave segment between the connector lane egress, west of Newport Center Drive, and the Newport Center Drive and I-95 ramp terminal intersection.



1.0 Background

There are three major documents relevant to this memo:

- I-95 at SW 10th St PD&E Study from south of SW 10th Street to north of Hillsboro Boulevard (2020)
 - FPID 436964-1
- SW 10th Street Connector PD&E Study from Military Trail to west of Powerline Road (2020)
 - FPID 439891-1
- SW 10th Street at I-95 Alternatives Analysis Memorandum (2018)
 - FPID N/A, supporting documentation for 436964-1 & 439891-1

It should be noted that the entire analysis in this 2021 memo is based on the 2040 Build volume set for the SW 10th Street without Powerline Road Ramps Alternative (sometimes referred to as Scenario C2), which corresponds to Alternative 1 in the SW 10th Street PD&E and Alternative 2A in the I-95 PD&E.

1.1 SW 10th Street & I-95 PD&E Studies

Both PD&E studies include Newport Center Drive as an analysis intersection. As of 2020, both studies documented the RCUT at Newport Center Drive as the Build Alternative. This memo, written in 2021, may result in the I-95 at SW 10th St PD&E Study revising the Build Alternative to include a full intersection opening for local SW 10th St at Newport Center Dr.

In the SW 10th Street Connector PD&E Study, Vissim modeling was conducted where the weaving movement from the connector lane egress, west of Newport Center Drive, to the eastbound left-turn at SW 12th Ave/Newport Center Drive was **NOT** permitted.

1.2 SW 10th Street Alternatives Analysis Memo

In 2018, an alternatives analysis (**memorandum attached**) was conducted for SW 10th Street to determine the optimal managed lane ingress and egress configurations for Design Year 2040.

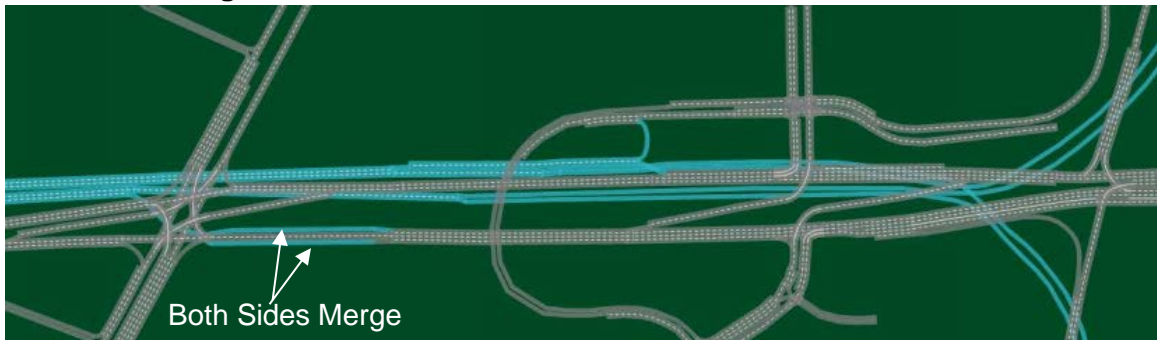
In the SW 10th Street Connector PD&E Study, Vissim modeling was conducted where the weaving movement from the connector lane egress, west of Newport Center Drive, to the eastbound left-turn at SW 12th Ave/Newport Center Drive **WAS** permitted, based origin on destination volumes (OD) described later.

As part of the analysis, each of the ingress/egress configurations assumed the RCUT configuration at the SW 12th Ave/Newport Center Drive intersection. The egress configurations considered for the location west of Newport Center Drive were:

- **Inside Merge**



- **Both Sides Merge**



- **Outside Merge**



To compare the egress configurations, the 2018 Alternatives Analysis Memorandum reported the overall 2040 model delay and the required minimum lane changes. The conclusion of the memo was that the Both Sides Egress Merge provided the best operating conditions. However, the Outside Egress Merge was the option carried into the SW 10th Street Connector and I-95 at SW 10th St PD&E studies.

2.0 Methodology

To evaluate the eastbound connector lane egress movement to the eastbound left-turn at SW 12th Ave/Newport Center Drive, the OD demand was calculated using the 2018 Alternatives Analysis data, shown in **Figure 1**. The relative percentage distribution per movement was applied to the “without Powerline Road Ramps” 2040 volume set (Alternative 1 in the SW 10th Street Connector PD&E Study and Alternative 2A in the I-95/SW 10th St PD&E).

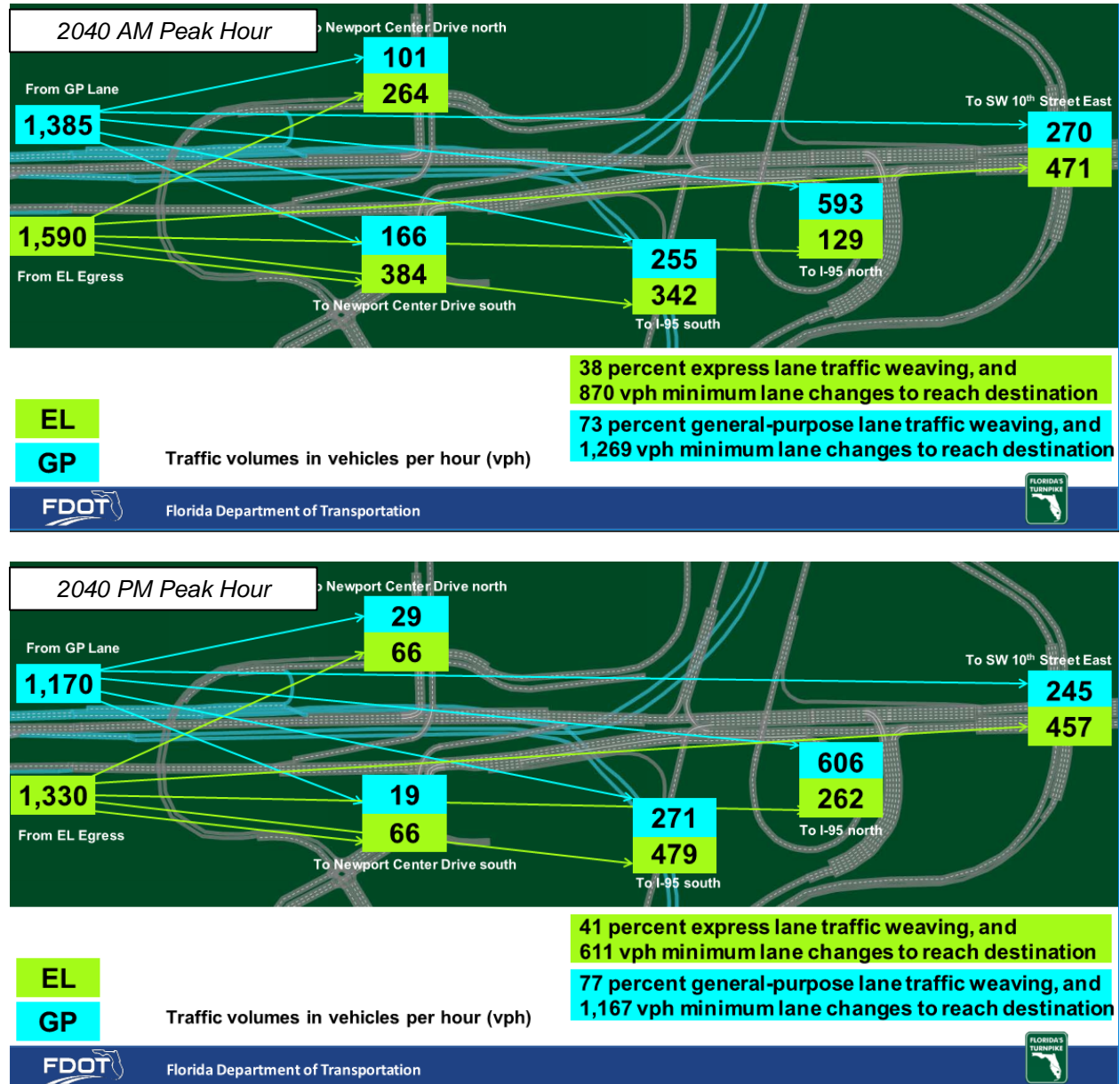


Figure 1. 2018 Alternatives Analysis Memo Origin-Destination Data

To evaluate the overall eastbound connector lane egress weave to the SW 12th Ave/Newport Center Drive and the I-95 ramp terminal intersections, Vissim version 8.00-15 was used. The Vissim network was previously calibrated and driver behavior inputs are consistent with the Vissim model used in the 2018 Alternatives Analysis Memorandum and in the 2020 SW 10th Street Connector PD&E Study. The simulation time was 3 hours with Measures of Effectiveness (MOEs) reported for the peak hour. The MOEs, listed below, were selected based on the MOEs used in the ongoing SW 10th Street/I-95 PD&E studies and in discussions with FDOT.

- Vissim – Average Speeds; Served Volume; Delay, LOS, and Queue Length

3.0 Alternative

The alternative analyzed was a full intersection opening at Newport Center Drive, as shown in **Figure 2**. Surface level streets are displayed in the grey and gold/brown color. The elevated 10th Street connector lanes are represented by the blue color. This configuration for Newport Center Drive includes lane modifications relative to the existing configuration.

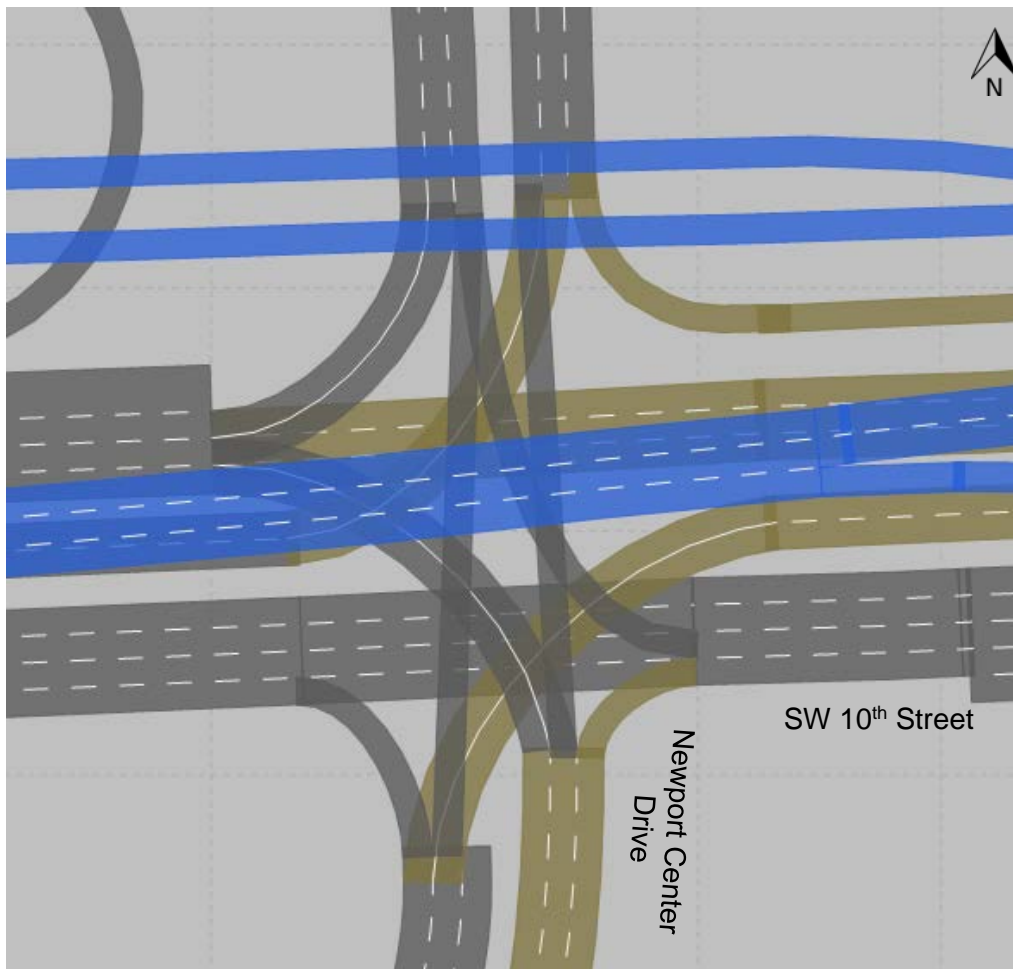


Figure 2. Full Intersection Opening Alternative for Newport Center Drive

4.0 Analysis Summary

The eastbound weave distance and 2040 segment volumes, as shown in **Figure 3**, are the same for the full intersection opening and the RCUT documented in the SW 10th Street Connector PD&E Study. The 2040 AM and PM peak hour turning movement volumes (TMVs) for the full intersection opening adjusts the RCUT northbound and southbound approaches to reflect the ability to turn left and through, shown in **Figure 4**. The full intersection TMVs were obtained from the source traffic forecasting conducted by Florida's Turnpike Enterprise (FTE). Note the RCUT TMVs were also developed from the FTE forecasts, however, there are some minor differences due to the RCUT configuration assuming some volume uses the SW 12th Ave slip ramp.

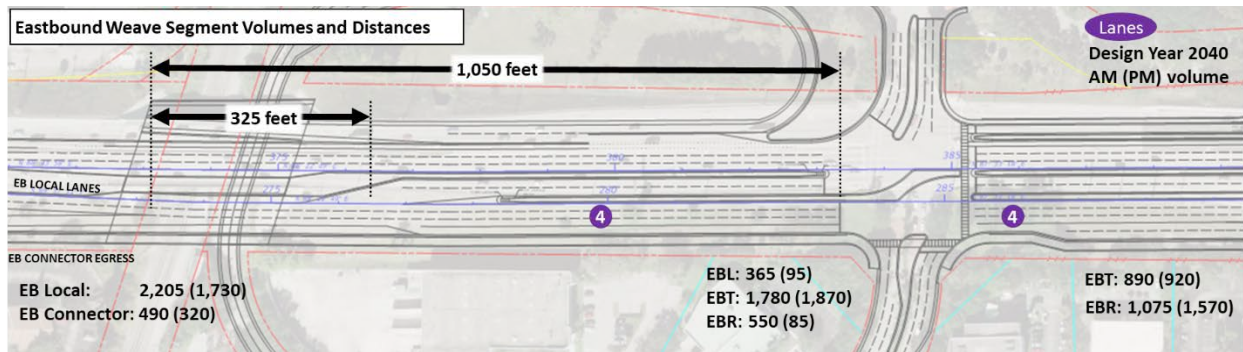


Figure 3. Eastbound Weave Segment Volumes and Distances along SW 10th Street

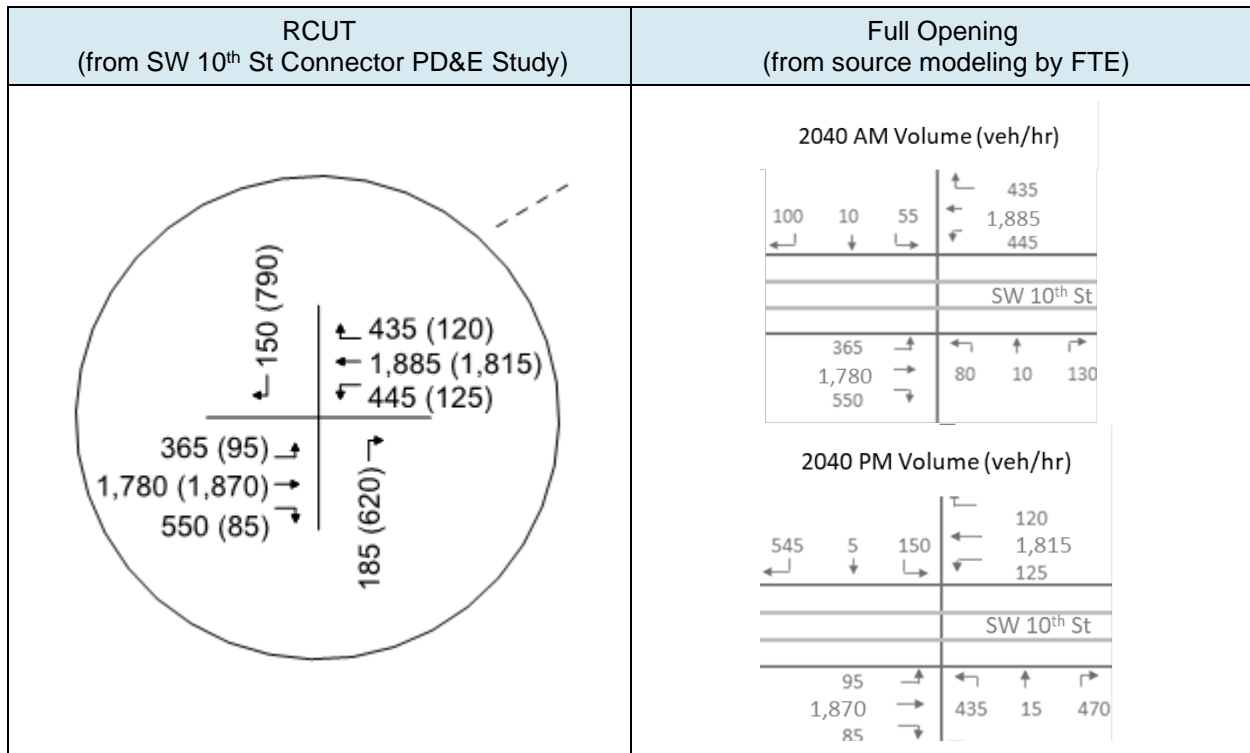


Figure 4. Newport Center Drive 2040 Design Year Traffic Volumes

4.1 Eastbound Weaving Movement from the Connector Lane Egress to the Eastbound Left-Turn at SW 12th Ave/Newport Center Drive

The relative OD percentages for the general purpose (GP) and express lane¹ (EL) egress were calculated for the AM peak hour using the volumes provided in the 2018 Alternatives Analysis Memo. The AM peak hour was chosen for analysis since the eastbound egress and eastbound left turn volumes at SW 12th Ave/Newport Center Drive were higher than during the PM peak hour. The OD percentages for both the GP and EL egress are shown below in **Figure 5**.

The OD percentages were applied to the eastbound EL egress and GP volumes from the SW 10th Street Connector PD&E Study to determine the eastbound left-turn volumes that originate from the EL egress or GP lanes. As shown in **Figure 6**, approximately 80 vehicles would originate from the EL egress.

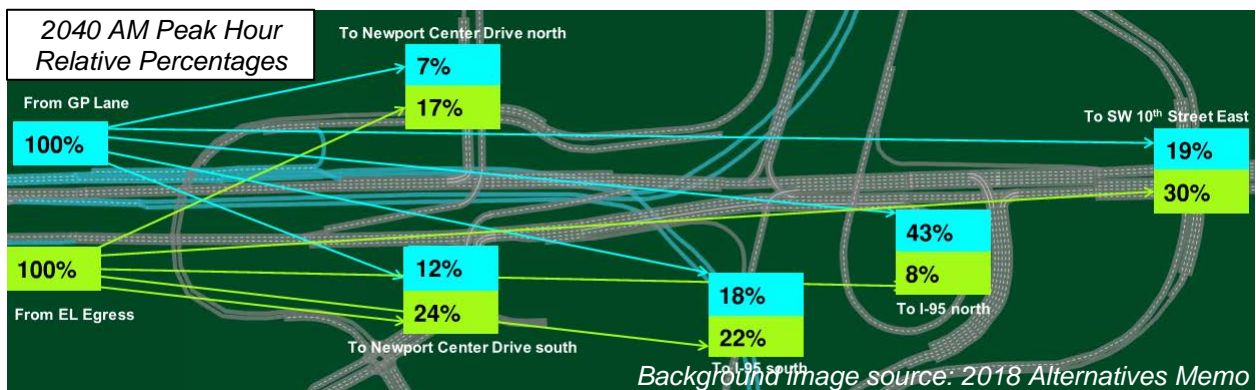


Figure 5. Relative OD Percentages from the Alternatives Analysis Memo

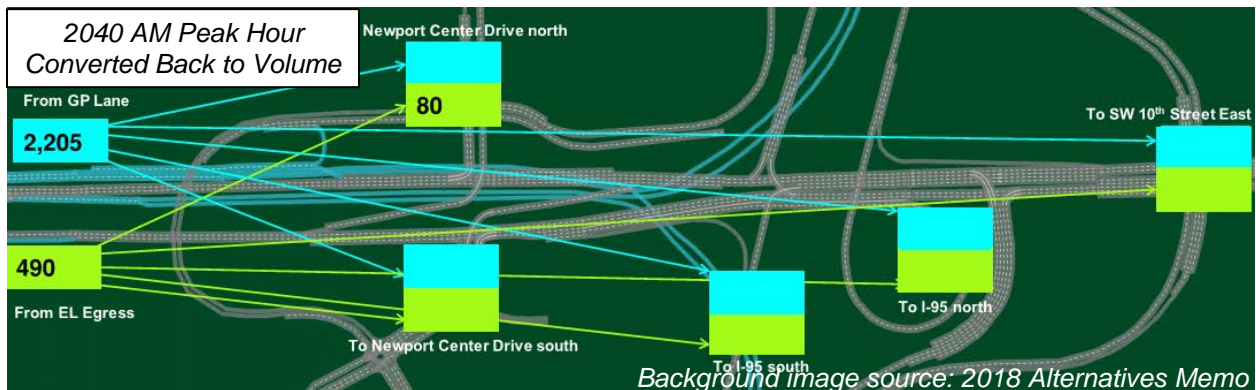


Figure 6. Origin-Destination Volumes for the Eastbound Left-Turn from EL Egress

¹ Note that “express lanes” here refers to the SW 10th Street connector lanes. In 2018, the terminology for the SW 10th Street connector lanes was “express lanes”.

Given the potential demand for the weaving movement and the short distance of 325 feet to complete the movement, the FDOT Access Management Handbook and the Florida Design Manual (FDM) were reviewed for minimum weaving distance guidance prior to operational analysis.

The FDOT Access Management Handbook states that “drivers may make erratic maneuvers in areas where there is limited separation between the off-ramp and the median opening”. The desirable weaving distance to overcome erratic behavior is shown in **Figure 7**, where the weaving distance for two lanes of traffic is 800 to 1,600 feet, assuming a weaving segment average speed of 34 to 45 miles per hour (mph). The Handbook does state that the weave section may be as low as 400 feet if average speeds can be reduced to 35 mph. However, given that the connector lane egress is a free-flow movement with a design and posted speed of 35 mph (based on the SW 10th Street typical section package), 40 mph would be expected as the free-flow speed. Further, the 325-foot distance violates the Florida Administrative Code standard that connection spacing be 440 feet for the context classification of this roadway (C3R).

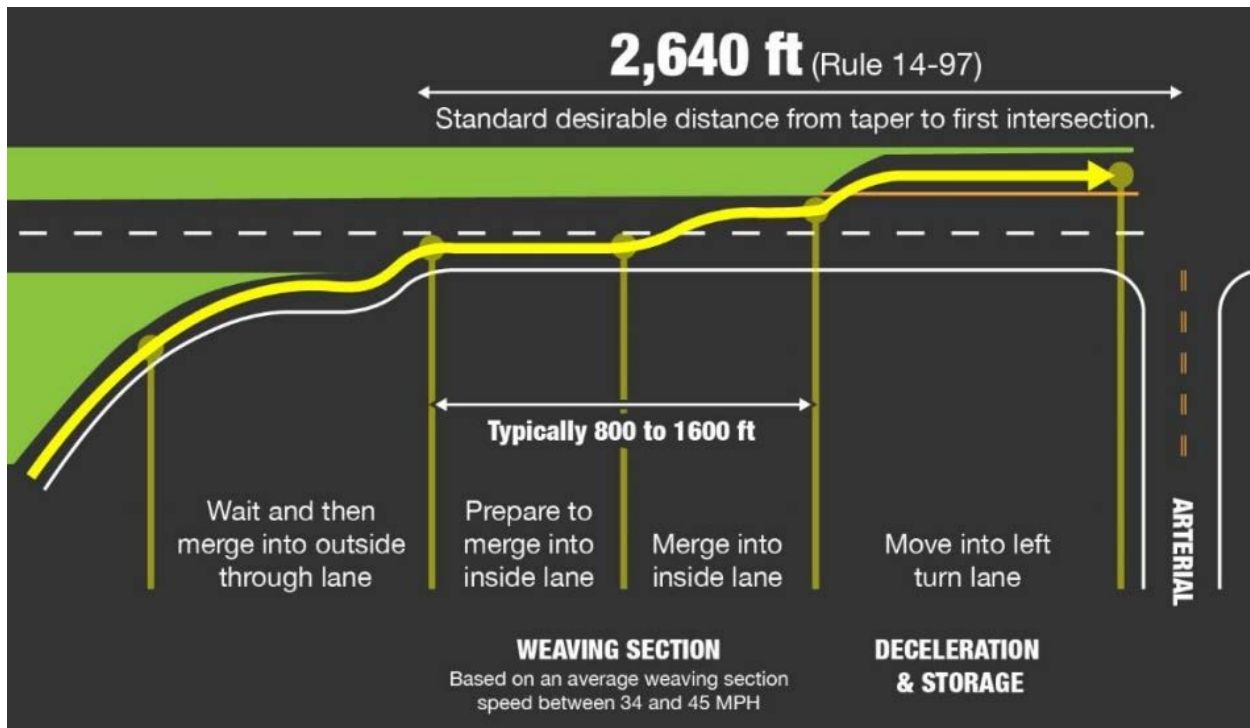


Figure 7. FDOT Access Management Handbook Weaving Distance Guidance

The FDM also includes standard geometric details at ingress and egress locations. In FDM Exhibit 211-4, shown in **Figure 8**, there is a 1,000-foot minimum distance specified as the starting weave length.

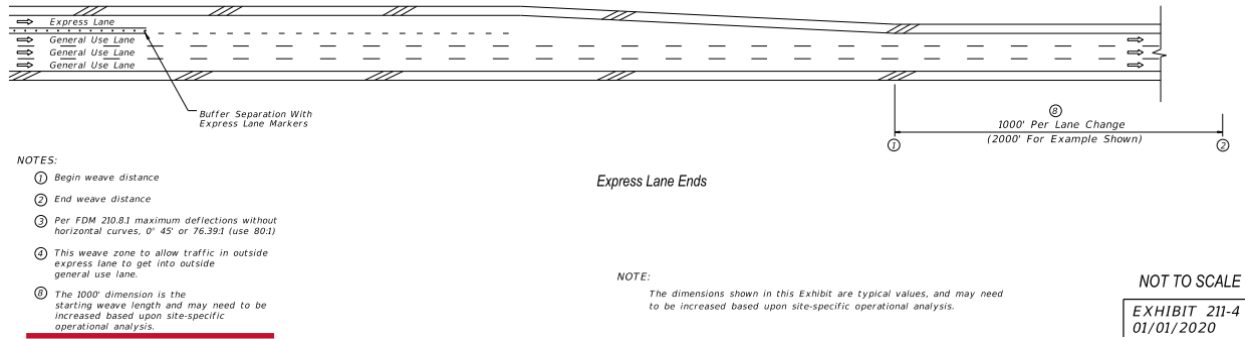


Figure 8. FDM Exhibit 211-4 (Begin Express Lanes Typical Egress)

To confirm that a short weave distance was not operationally viable, a Vissim simulation for the AM peak hour was run, as shown in **Figure 9**. The simulation resulted in vehicles being forced to come to a full stop in the weave segment to allow for the eastbound egress to eastbound left-turn to be completed.

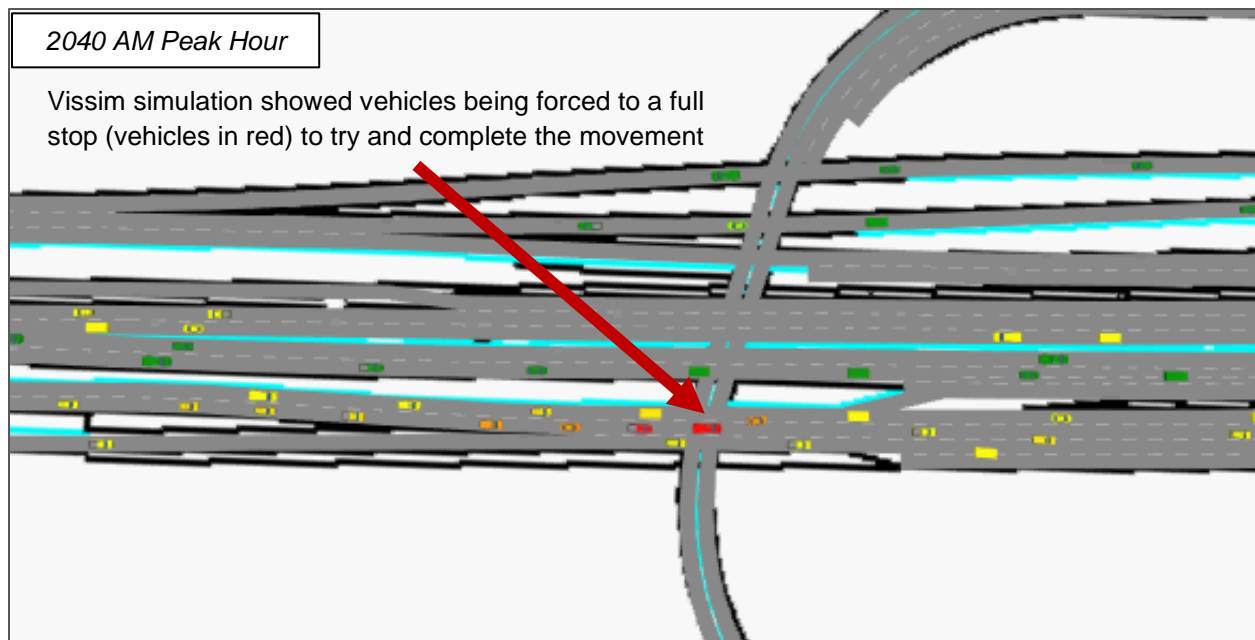


Figure 9. Vissim Simulation Snapshot for the Eastbound Egress to Left-Turn Movement

Overall, based on the FDOT guidance and operational simulation showing that vehicles must make a full stop in the weave segment, the eastbound egress weaving movement to the eastbound left-turn at SW 12th Ave/Newport Center Drive should not be permitted. This restriction is consistent with the SW 10th Street Connector PD&E study modeling effort. The vehicles exiting the eastbound connector lane, with a destination of northbound SW 12th Ave/Newport Center Drive, would be expected to make an eastbound right-turn at Newport Center Drive and use the Newport Center Drive and the SW 12th Avenue loop road below SW 10th St, to reach the north side (SW 12th Ave).

4.2 Eastbound Weave Segment from the Connector Lane Egress to the SW 12th Ave/Newport Center Drive and the I-95 Intersections

The overall eastbound weave segment operations were evaluated based on eastbound queueing in the weave segment and eastbound segment speeds. These metrics offer comparison to the previous efforts in the PD&E studies where the RCUT configuration was assumed. The intersection of Newport Center Drive was modeled in Vissim as a connected system with the I-95 interchange ramps for synchronization purposes with the cycle lengths and offsets. Full interchange results, including delay and LOS, can be found as part of the Vissim analysis summary presentation in the **Attachments**.

4.2.1 Eastbound Weaving Volumes

The weaving OD volumes used in this Newport Center Drive full intersection analysis are displayed in **Figure 10**. Based on the insufficient weaving distance between the connector lane egress and the eastbound left-turn at SW 12th Ave/Newport Center Drive, as identified in *Section 4.1*, the OD volume is assumed as zero for that movement. These OD volumes are consistent with the SW 10th Street Connector PD&E study.

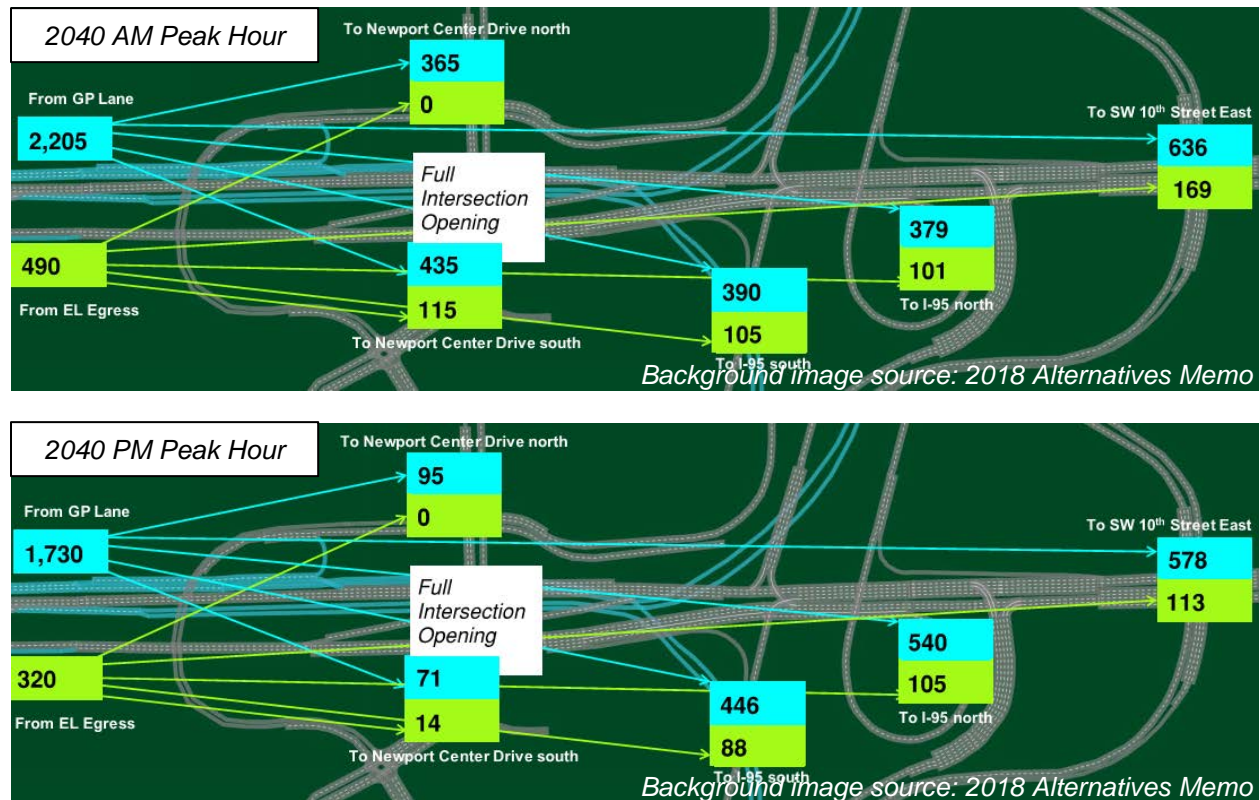


Figure 10. Weaving OD Volumes Used in Newport Center Drive Full Intersection Analysis

4.2.2 Eastbound Queueing in the Weave Segment

The eastbound queueing at the Newport Center Drive intersection is expected to interact with the ability to make weaving maneuvers by reducing the effective weave distance. While the entire segment length from the eastbound egress to the Newport Center Drive eastbound stop bar is 1,025 feet, that distance may be effectively reduced by a queue. The eastbound intersection queues are shown in **Tables 1** and **2**. The eastbound queueing for the full intersection configuration is similar to the RCUT configuration, although delays differ due to signal timing differences. The SW 10th Street PD&E utilized a 180 second cycle length and the Newport Center Drive alternatives analysis used a 150 second cycle length. Operational intersection results for the rest of the Newport Center Drive movements and the I-95 interchange are provided in the **Attachments**.

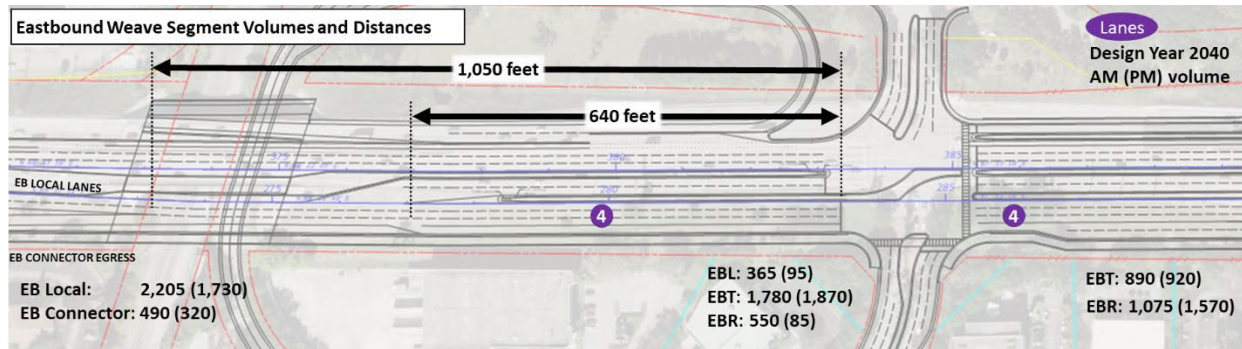
Table 1. Eastbound SW 10th Street at Newport Center Drive – 2040 AM Peak Hour Results

SW 10 th Street Intersection Location	Mvmnt	SW 10 th St PD&E (Newport as RCUT)				Alternative (Newport as Full Intersection)			
		Served Volume	Max Queue (ft)	Delay and LOS		Served Volume	Max Queue (ft)	Delay and LOS	
				Mvmnt	App			Mvmnt	App
Newport Center Dr Eastbound	L	359	315	84.0 (F)	16.9 (B)	345	281	66.2 (E)	27.8 (C)
	T	1,776	640	6.0 (A)		1,717	640	20.5 (C)	
	R	545	640	9.0 (A)		504	640	26.1 (C)	

Table 2. Eastbound SW 10th Street at Newport Center Drive – 2040 PM Peak Hour Results

SW 10 th Street Intersection Location	Mvmnt	SW 10 th St PD&E (Newport as RCUT)				Alternative (Newport as Full Intersection)			
		Served Volume	Max Queue (ft)	Delay and LOS		Served Volume	Max Queue (ft)	Delay and LOS	
				Mvmnt	App			Mvmnt	App
Newport Center Dr Eastbound	L	92	164	55.0 (D)	6.9 (A)	91	109	73.4 (E)	34.5 (C)
	T	1,837	613	5.0 (A)		1,846	617	32.5 (C)	
	R	83	613	5.0 (A)		82	617	36.4 (D)	

The maximum eastbound through queue for the full intersection opening occurs in the AM peak hour and is 640 feet, which is the approximate distance shown below.



4.2.3 Eastbound Segment Speeds

The eastbound queue results are supported by the segment speeds, given that a queue is not expected to be present at all times. The segment speeds with Newport Center Drive as a full intersection versus Newport Center Drive as an RCUT is shown in **Figures 11** and **12**. The eastbound segment speeds remain similar, primarily experiencing 30-45 mph immediately entering the weave, and reducing to 15-30 mph approaching the Newport Center Drive signal. It is also noted that the southbound exit ramp experiences additional slowdowns (<15 mph) in the Full Intersection option, which is indicative of increased queuing. Maximum ramp queues are summarized in the next **Section 4.2.4**.

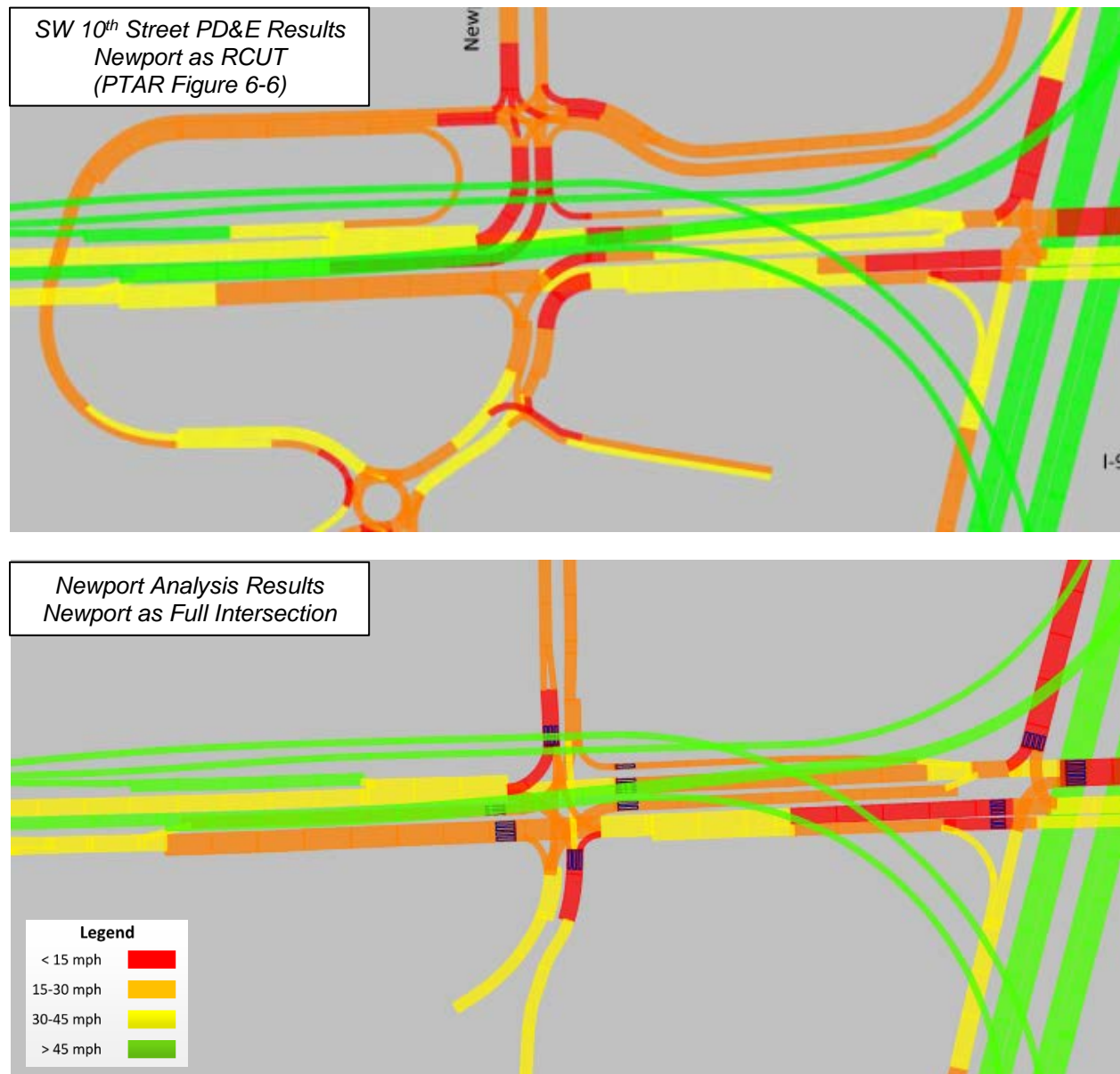


Figure 11. Eastbound SW 10th Street at Newport Center Drive – 2040 AM Peak Hour Segment Speeds

In the PM peak hour, the segment speeds entering the weave segment for the full intersection opening are also similar to the RCUT. However, the full intersection option experiences sustained 15-30 mph speeds through the majority of the weave segment, which is lower than the RCUT. This is primarily due to the shorter cycle length and increased side street signal time required to serve Newport Center Drive, which has higher northbound and southbound volumes in the PM peak hour. It is also noted that the southbound exit ramp experiences additional slowdowns (<15 mph) in the Full Intersection option, which is indicative of increased queuing. Maximum ramp queues are summarized in **Section 4.2.4**.

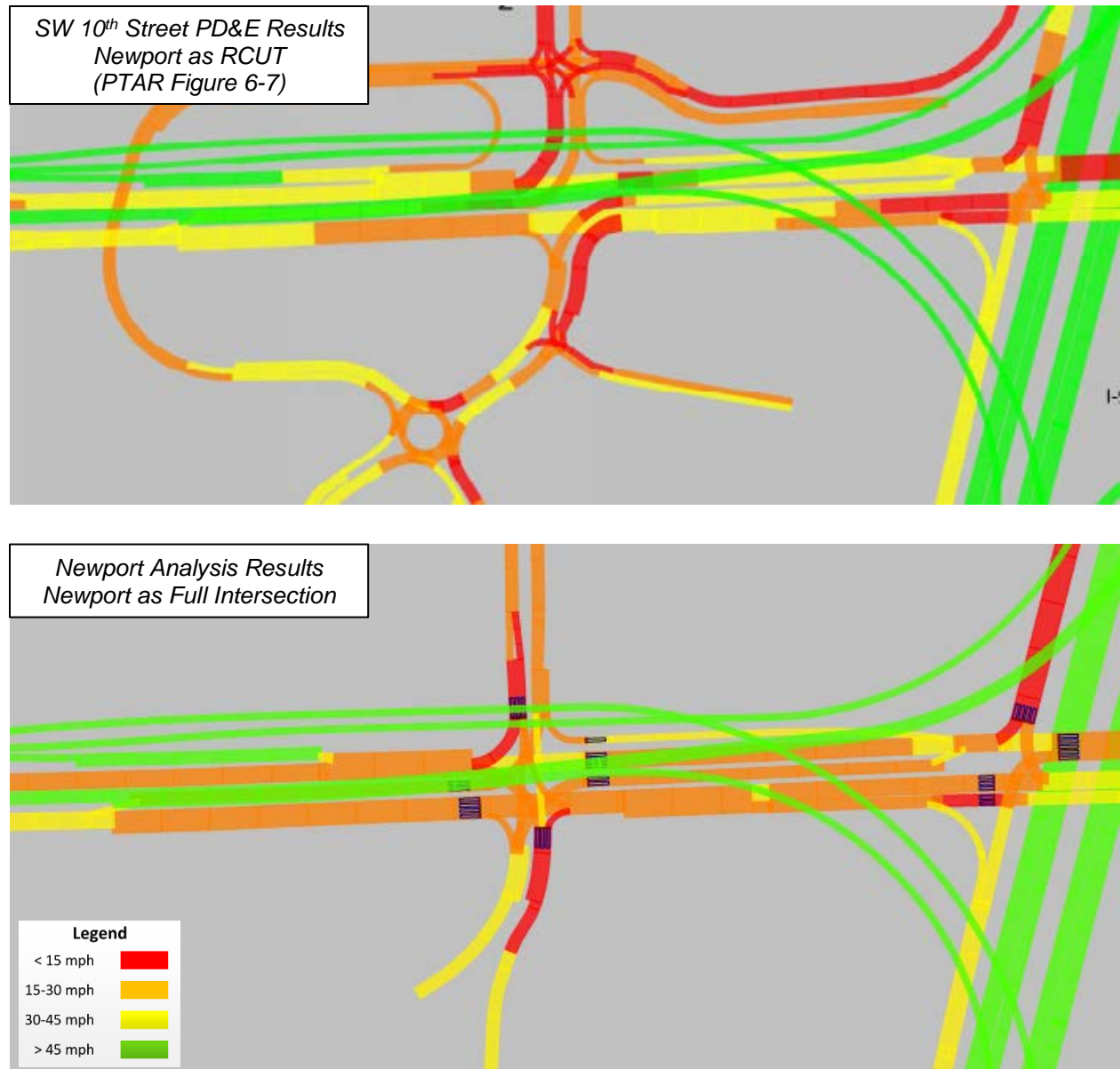


Figure 12. Eastbound SW 10th Street at Newport Center Drive – 2040 PM Peak Hour Segment Speeds



4.2.4 I-95 Interchange Exit-Ramp Terminal Queues

The southbound and northbound exit ramp queues are compared in **Table 3** and **Table 4**. The queues for the Full Intersection option remain within the available storage distance for all but one movement – the southbound approach in the PM peak hour. For this movement, the maximum queue is approximately 1 car length beyond the storage distance (648 feet of queue versus 625 feet of storage). Visual observation of the traffic simulation indicated this maximum queue occurred only rarely during the peak hour. Also, for this approach, there is an additional 1,000+ feet beyond the 625-foot storage bay distance, prior to the physical I-95 gore point. Given that configuration and the low occurrence of the maximum queue relative to an average queue, gore modifications are not anticipated. Full interchange results, including delay and LOS, can be found as part of the Vissim analysis summary presentation in the **Attachments**.

Table 3. I-95 at SW 10th Street Interchange Exit Ramps – 2040 PM Peak Hour Results

I-95 Ramps at SW 10 th Street	Mvmnt	SW 10 th St PD&E (Newport as RCUT)			Alternative (Newport as Full Intersection)		
		Served Volume	Max Queue (ft)	Storage Distance (ft)	Served Volume	Max Queue (ft)	Storage Distance (ft)
Northbound Exit Ramp	NBL	520	232	425	524	223	425
	NBR	691	294		676	276	
Southbound Exit Ramp	SBL	295	233	625	281	391	625
	SBR	572	233		583	391	

Table 4. I-95 at SW 10th Street Interchange Exit Ramps – 2040 PM Peak Hour Results

I-95 Ramps at SW 10 th Street	Mvmnt	SW 10 th St PD&E (Newport as RCUT)			Alternative (Newport as Full Intersection)		
		Served Volume	Max Queue (ft)	Storage Distance (ft)	Served Volume	Max Queue (ft)	Storage Distance (ft)
Northbound Exit Ramp	NBL	768	364	425	770	298	425
	NBR	479	228		465	212	
Southbound Exit Ramp	SBL	409	333	625	396	648	625
	SBR	835	333		847	648	

5.0 Conclusion

The purpose of this memo was to document:

- (1) whether the weaving movement from the connector lane egress, west of SW 12th Ave/Newport Center Drive, to the eastbound left-turn at SW 12th Ave/Newport Center Drive is feasible based on traffic demand and design guidance, and
- (2) how the full intersection opening affects traffic operations for the overall eastbound weave segment between the connector lane egress, west of Newport Center Drive, and the Newport Center Drive and I-95 intersections.

In response to those two points, **key findings** of the traffic operational analysis conducted for the Newport Center Drive Full Intersection Opening and its impact on the eastbound weave components are:

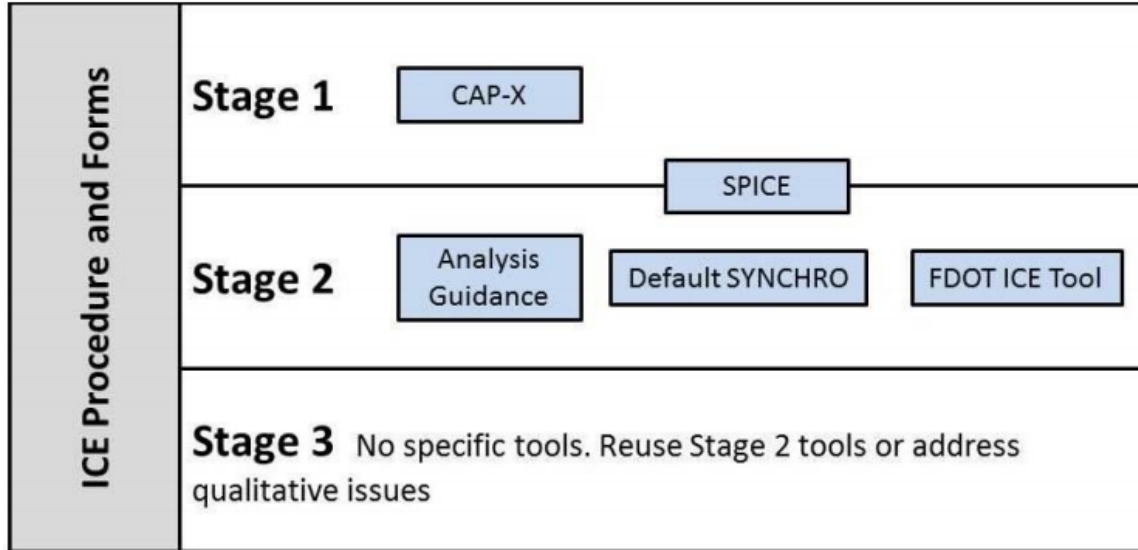
- (1) The weaving movement demand from the connector lane egress to the eastbound left-turn at SW 12th Ave/Newport Center Drive could be expected to be approximately 80 vehicles, if permitted. However, it is recommended that this weave movement be physically prohibited due to design concerns and inability to meet recommended minimum weave distances in the FDOT Access Management Handbook and the FDM.
 - a. This memo is anticipated to result in a design change to the previous Build Configuration in the PD&E Studies. The design change would shift the Newport Center Drive eastbound left-turn turn bay access west (upstream of the connector lane egress) to physically prohibit connector lane traffic from being able to access to turn bay.
- (2) When compared to the RCUT, the full intersection opening at Newport Center Drive results in similar traffic operations for the eastbound queueing and segment speeds. Therefore, the overall eastbound weave segment, from the connector lane egress to the Newport Center Drive intersection and the I-95 interchange is not anticipated to be adversely impacted when compared to the RCUT.

It should also be noted that while no adverse impacts are expected at the I-95 ramp terminals with the Full Intersection configuration, the RCUT configuration does provides residual capacity beyond the design year, as supported by the more favorable speeds and queuing shown in **Figures 11 and 12**.

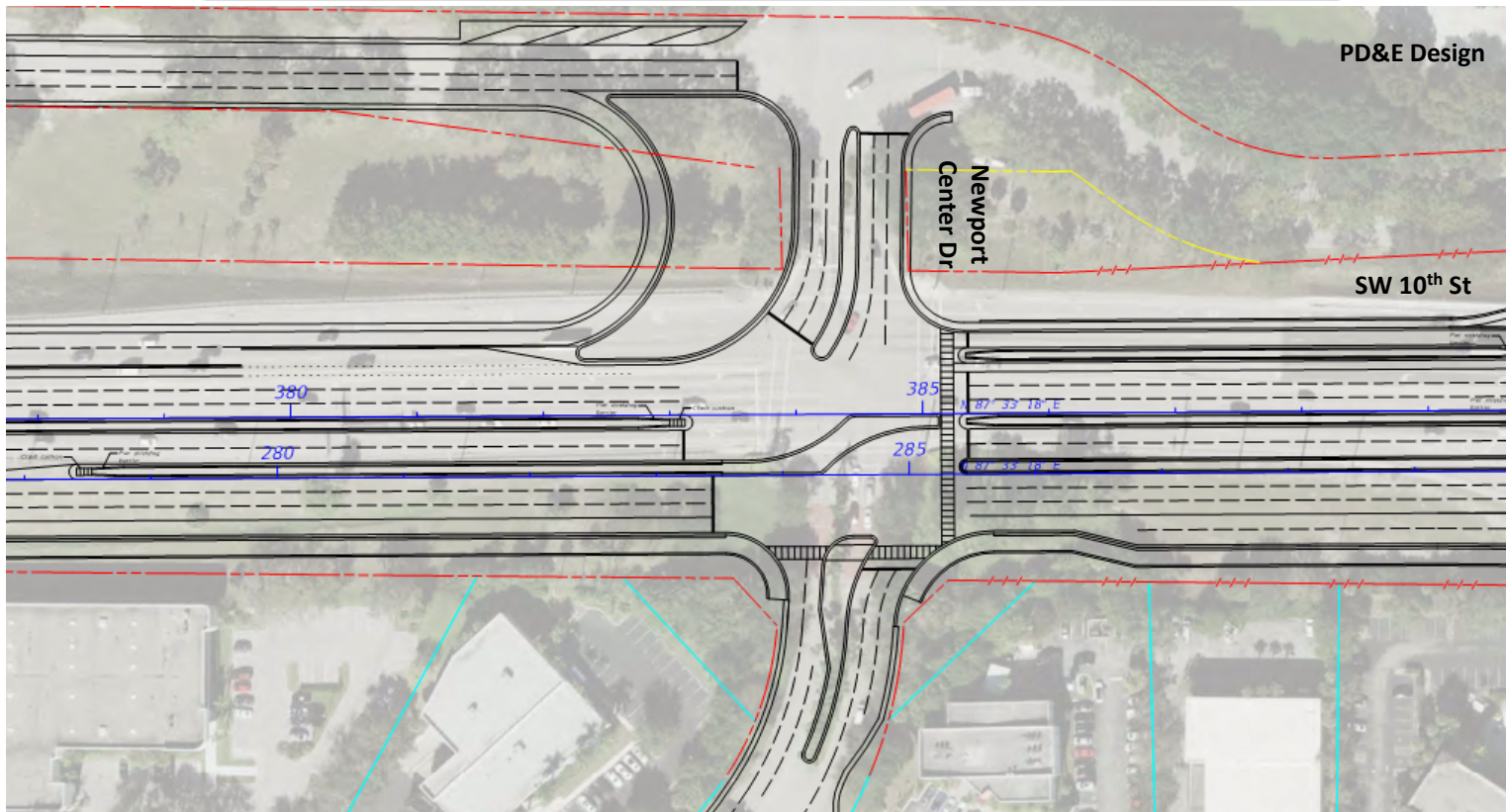


Attachments

SW 10th Street CDC - Newport Center Drive Intersection Control Evaluation (ICE)



← We are here





DRAFT

SW 10th Street CDC - Newport Center Drive Intersection Control Evaluation (ICE)

November 2020

2016 AM



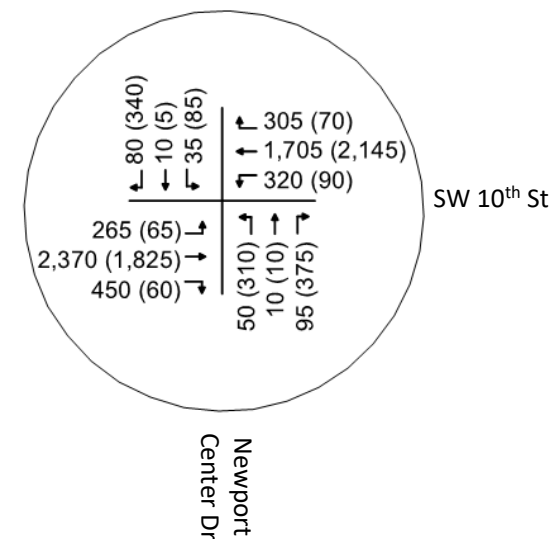
TYPE OF INTERSECTION	Overall V/C Ratio	V/C Ranking	Multimodal Score	Pedestrian Accommodations	Bicycle Accommodations	Transit Accommodations
Signalized Restricted Crossing U-Turn E-W	0.46	1	6.3	Good	Good	Fair
Median U-Turn E-W	0.56	2	6.3	Good	Good	Fair
Quadrant Roadway N-W	0.57	3	4.4	Fair	Fair	Fair
Partial Median U-Turn E-W	0.58	4	6.3	Good	Good	Fair
Quadrant Roadway S-W	0.59	5	4.4	Fair	Fair	Fair
Traffic Signal	0.83	6	4.8	Fair	Fair	Good
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Notes:

- Existing Configuration Assumed
- While some configurations listed may not be geometrically feasible, they were included for this initial capacity screening comparison.

2040 AM(PM) Volume (veh/hr)



2016 PM



TYPE OF INTERSECTION	Overall V/C Ratio	V/C Ranking	Multimodal Score	Pedestrian Accommodations	Bicycle Accommodations	Transit Accommodations
Median U-Turn E-W	0.55	1	6.3	Good	Good	Fair
Signalized Restricted Crossing U-Turn E-W	0.56	2	6.3	Good	Good	Fair
Quadrant Roadway S-W	0.58	3	4.4	Fair	Fair	Fair
Quadrant Roadway N-W	0.64	4	4.4	Fair	Fair	Fair
Partial Median U-Turn E-W	0.75	5	6.3	Good	Good	Fair
Traffic Signal	1.09	6	4.8	Fair	Fair	Good
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SW 10th Street CDC - Newport Center Drive Intersection Control Evaluation (ICE)

November 2020

2040 AM

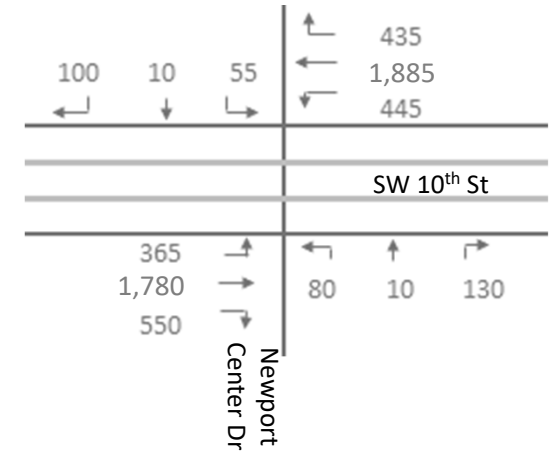


TYPE OF INTERSECTION	Overall V/C Ratio	V/C Ranking	Multimodal Score	Pedestrian Accommodations	Bicycle Accommodations	Transit Accommodations
Signalized Restricted Crossing U-Turn E-W	0.55	1	6.3	Good	Good	Fair
Quadrant Roadway N-W	0.62	2	4.4	Fair	Fair	Fair
Traffic Signal	0.66	3	4.8	Fair	Fair	Good
Median U-Turn E-W	0.71	4	6.3	Good	Good	Fair
Quadrant Roadway S-W	0.76	5	4.4	Fair	Fair	Fair
Partial Median U-Turn E-W	0.76	5	6.3	Good	Good	Fair
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Notes:

1. Ultimate 95 Ramp Configuration Assumed
2. While some configurations listed may not be geometrically feasible, they were included for this initial capacity screening comparison.

2040 AM Volume (veh/hr)



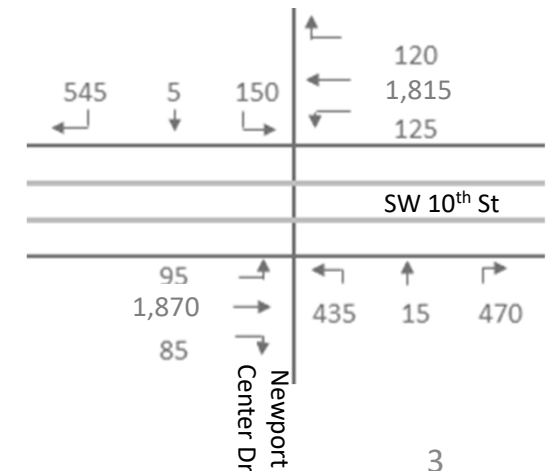
2040 PM



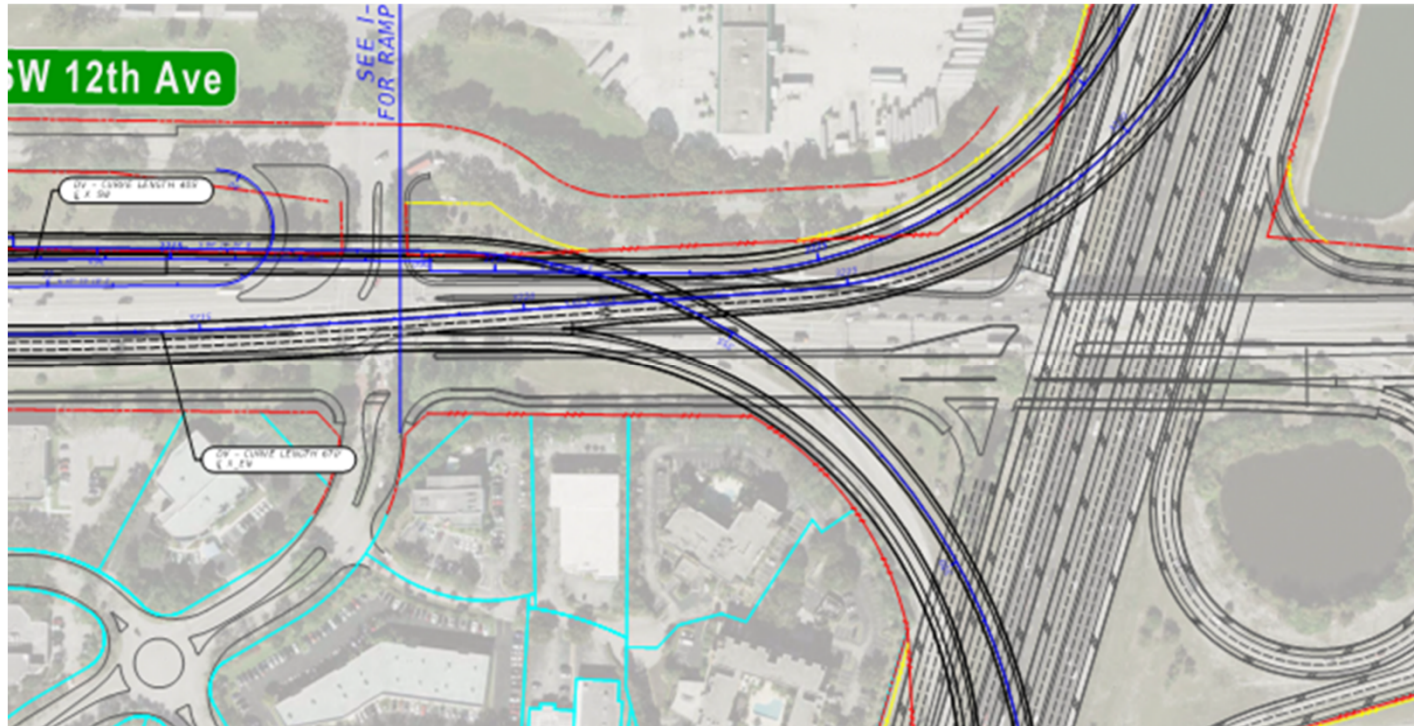
Fatal Flaw V/C >1

TYPE OF INTERSECTION	Overall V/C Ratio	V/C Ranking	Multimodal Score	Pedestrian Accommodations	Bicycle Accommodations	Transit Accommodations
Median U-Turn E-W	0.48	1	6.3	Good	Good	Fair
Signalized Restricted Crossing U-Turn E-W	0.58	2	6.3	Good	Good	Fair
Quadrant Roadway S-W	0.59	3	4.4	Fair	Fair	Fair
Quadrant Roadway N-W	0.69	4	4.4	Fair	Fair	Fair
Partial Median U-Turn E-W	0.79	5	6.3	Good	Good	Fair
Traffic Signal	1.12	6	4.8	Fair	Fair	Good
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2040 PM Volume (veh/hr)



December 17th, 2020



Agenda

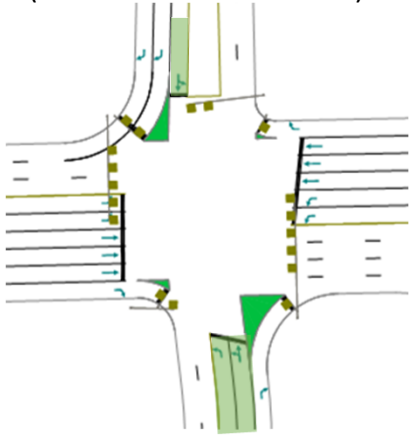
- Alternatives Analyzed
- Analysis Results
 - Intersection LOS and V/C Ratios
 - Critical Movement Queue Lengths
- Sensitivity Analysis Overview
- Recommendations and Next Steps (Vissim)

Analysis Approach

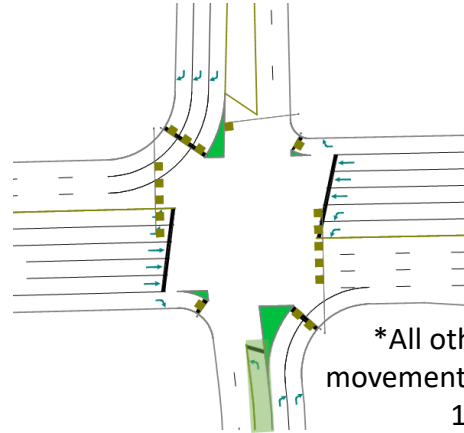
- 4 Newport Center Drive Alternatives vetted in Synchro for Design Year 2040
- Overall analysis includes: Newport Center Drive, I-95 West Ramp Terminal, I-95 East Ramp Terminal
- Corridor signals optimized and coordinated

SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Lane Configurations

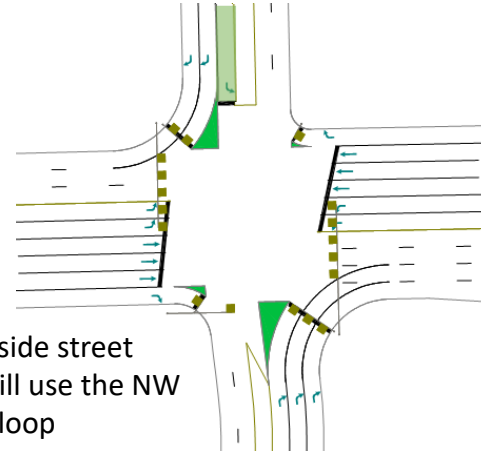
Alternative 1
Full Intersection Opening
(With Lane Modifications)



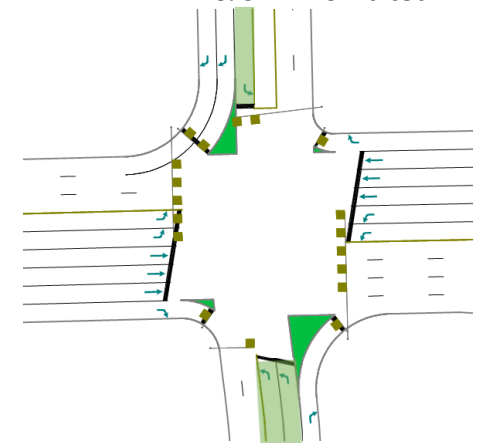
Alternative 2
Allow NBL Only*



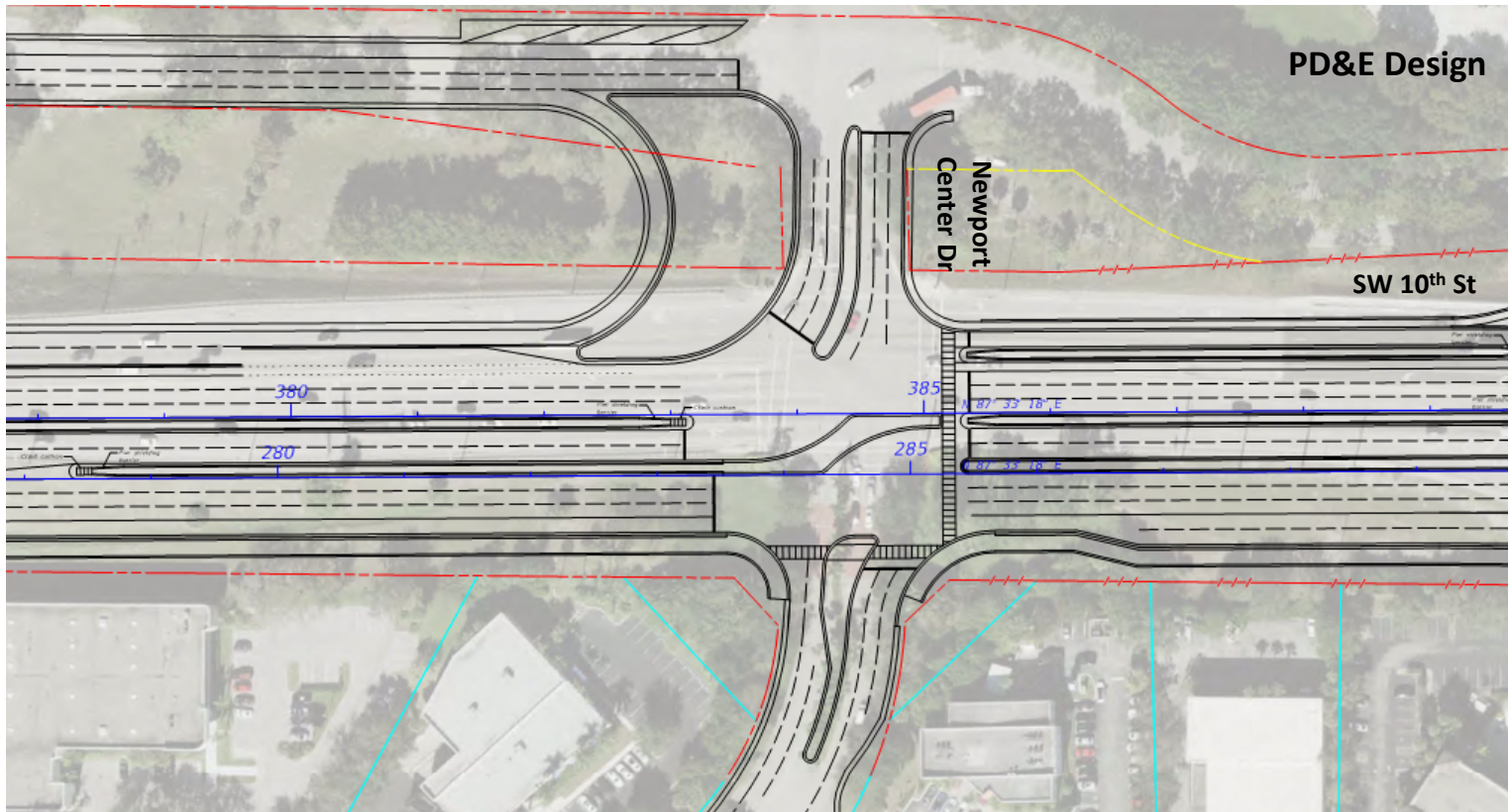
Alternative 3
Allow SBL Only*



Alternative 4
NBL & SBL Allowed, but
NBT & SBT Prohibited



*All other side street movements still use the NW 12th loop



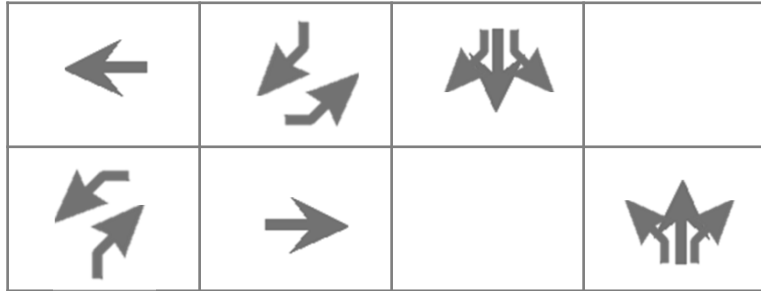
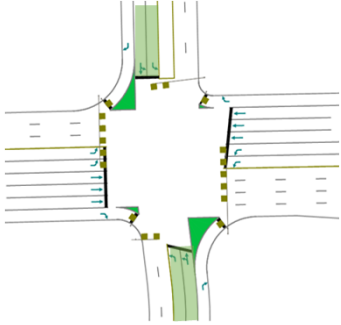
New movement permitted (compared to the RCUT)

SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis

Signal Phasing

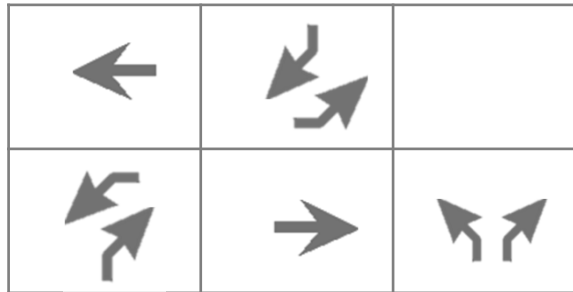
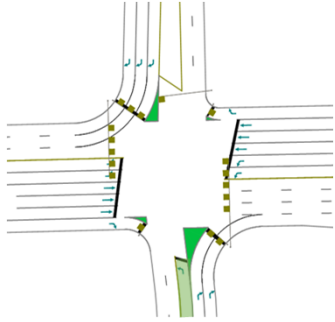
New movement permitted (compared to the RCUT)

Alternative 1
Full Intersection Opening (With Lane Modifications)



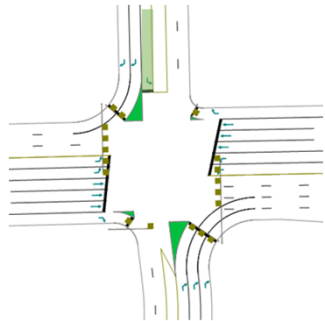
4-Phase Signal

Alternative 2
Allow NBL Only



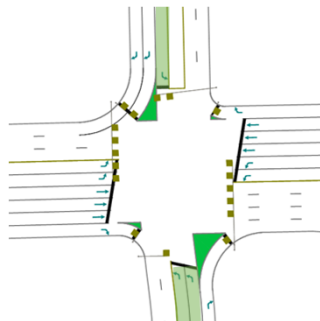
3-Phase Signal

Alternative 3
Allow SBL Only



3-Phase Signal

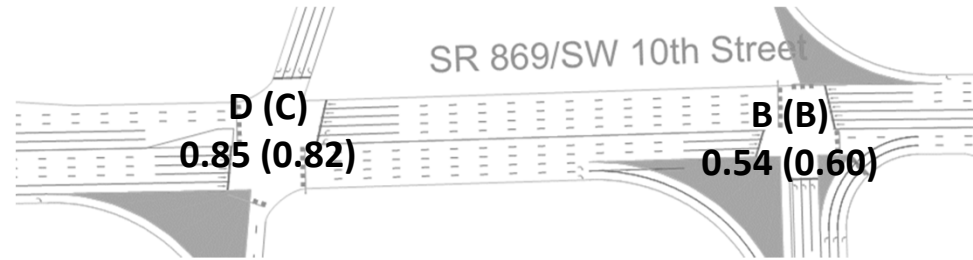
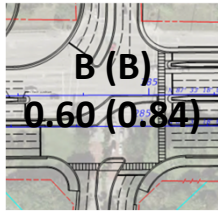
Alternative 4
NBL & SBL Allowed, but NBT & SBT Prohibited



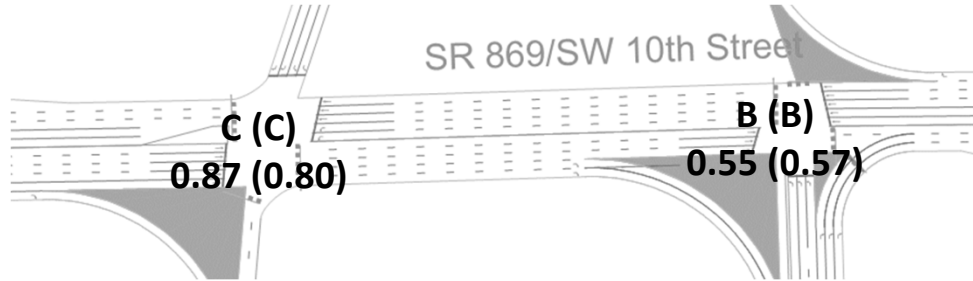
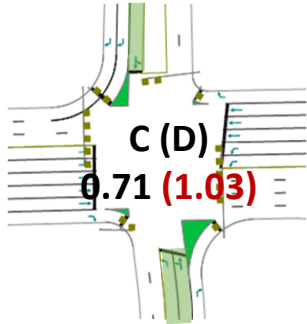
3-Phase Signal

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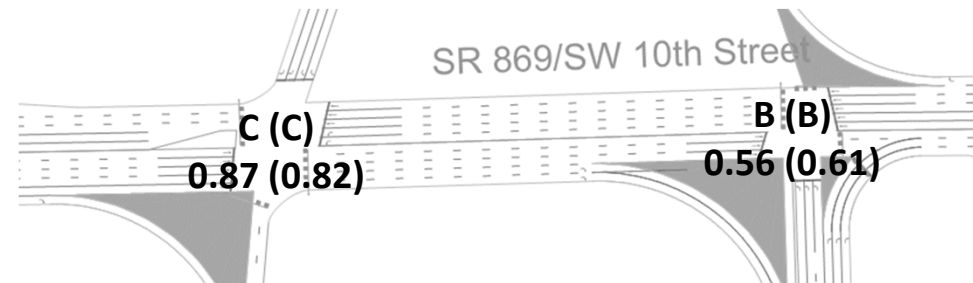
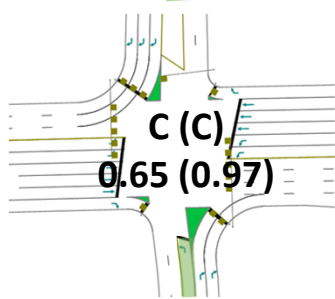
PD&E Design
RCUT



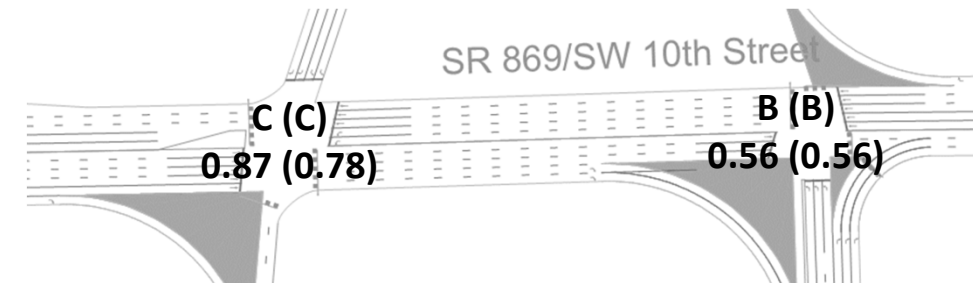
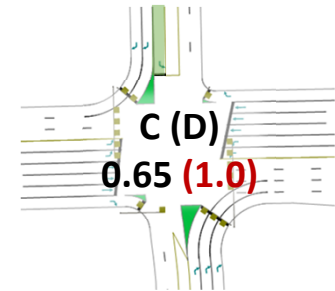
Alternative 1
Full Intersection
Opening
(With Lane
Modifications)



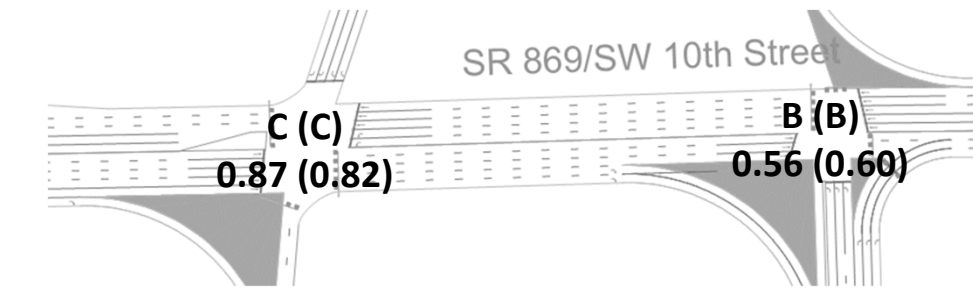
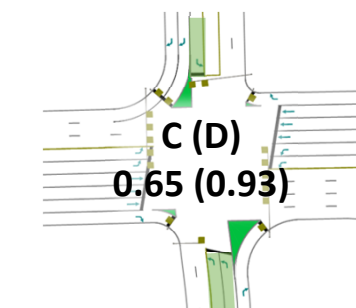
Alternative 2
Allow NBL Only



Alternative 3
Allow SBL Only



Alternative 4
NBL & SBL
Allowed,
but NBT & SBT
Prohibited



Legend & Notes

LOS: AM (PM)
V/C: AM(PM)

New movement permitted (compared to the RCUT)

2040 AM Volume (veh/hr)			
100	10	55	435
←	↓	↘	↖
			1,885
			445
SW 10 th St			
365	1,780	550	80
↖	→	↘	↖
			10
			130
Newport Center Dr			
2040 PM Volume (veh/hr)			
545	5	150	120
←	↓	↘	↖
			1,815
			125
SW 10 th St			
95	1,870	85	435
↖	→	↘	↖
			15
			470
Newport Center Dr			



SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis

2040 Critical Movement Queue Lengths

December 2020

DRAFT

Legend & Notes

= volume exceeds capacity, queue may be longer
 m = metered by upstream signal

New movement permitted (compared to the RCUT)

2040 AM Volume (veh/hr)

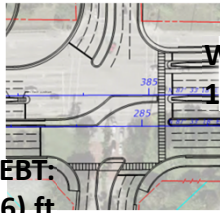
			↖	↗	
			435		
↖	100	10	1,885	↗	
			445		
			SW 10 th St		

2040 PM Volume (veh/hr)

			↖	↗	
			120		
↖	545	5	1,815	↗	
			125		
			SW 10 th St		

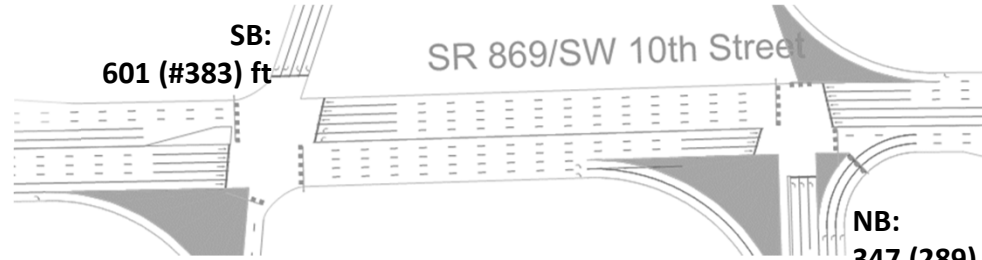
			↖	↗	
			435		
↖	95	85	1,870	↗	
			15		
			Newport 5		

PD&E Design RCUT



WBT: 186 (m193) ft

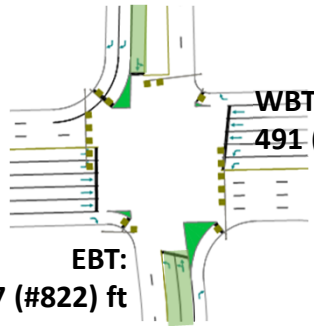
EBT: m351 (m186) ft



SB: 601 (#383) ft

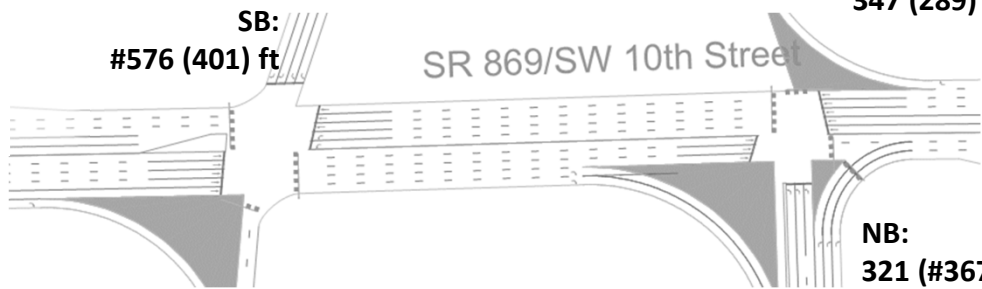
NB: 347 (289) ft

Alternative 1
 Full Intersection Opening (With Lane Modifications)



WBT: 491 (540) ft

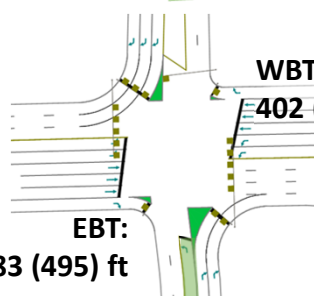
EBT: m667 (#822) ft



SB: #576 (401) ft

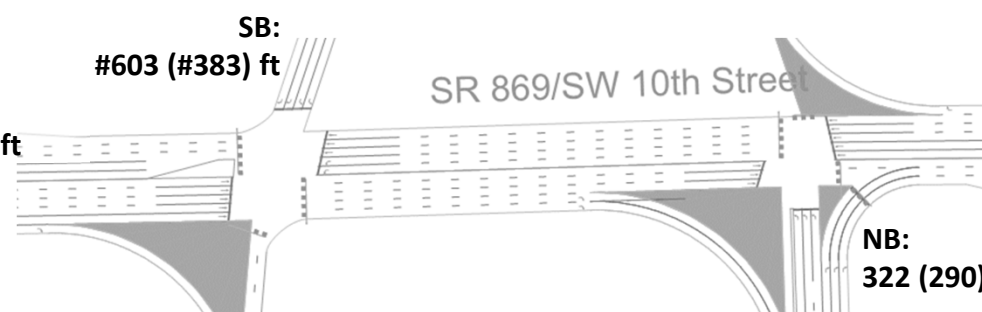
NB: 321 (#367) ft

Alternative 2
 Allow NBL



WBT: 402 (m#638) ft

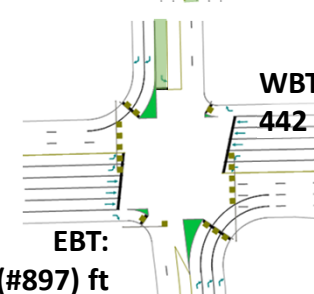
EBT: m483 (495) ft



SB: #603 (#383) ft

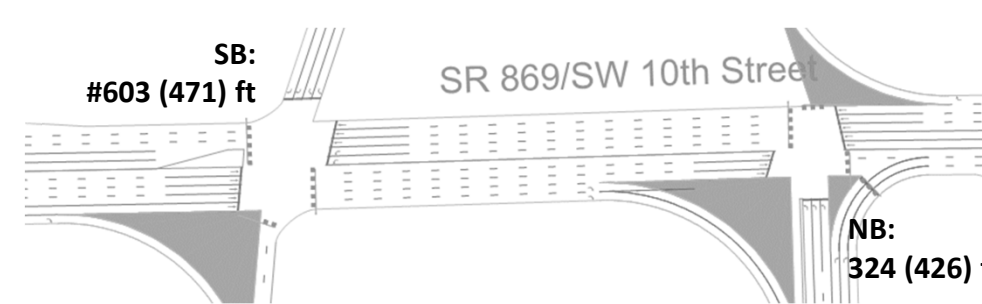
NB: 322 (290) ft

Alternative 3
 Allow SBL



WBT: 442 (633) ft

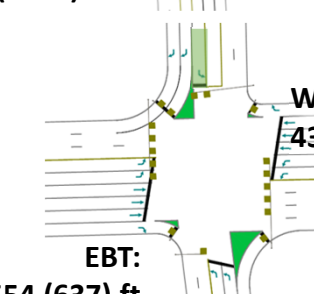
EBT: m560 (#897) ft



SB: #603 (471) ft

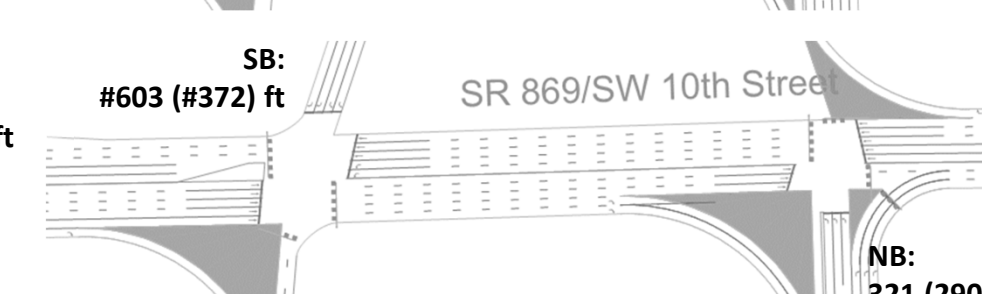
NB: 324 (426) ft

Alternative 4
 NBL & SBL Allowed, but NBT & SBT Prohibited



WBT: 433 (550) ft

EBT: m554 (637) ft



SB: #603 (#372) ft

NB: 321 (290) ft



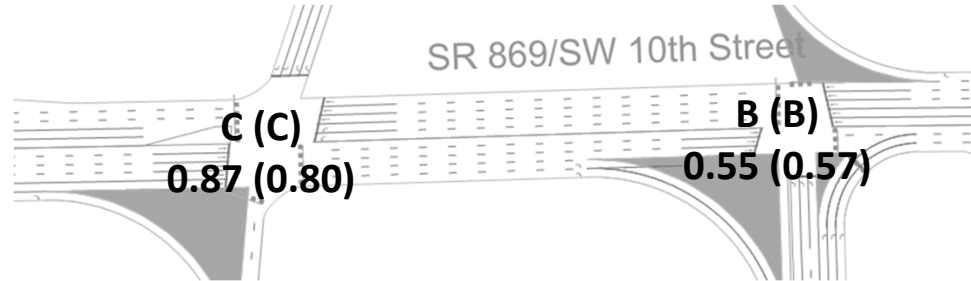
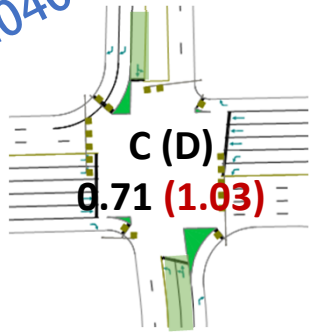
DRAFT

SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Sensitivity Analysis for FULL Intersection Alternative

December 2020

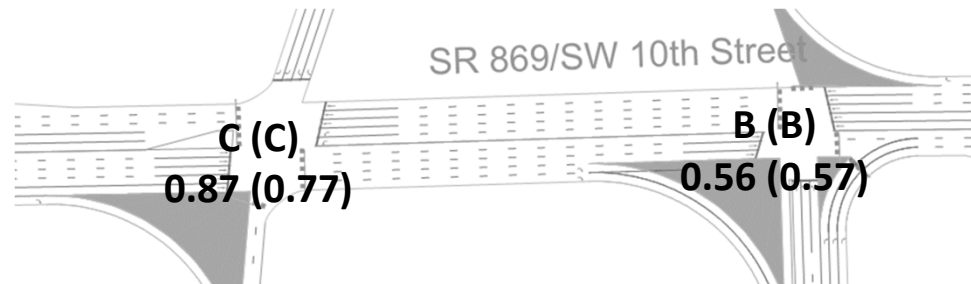
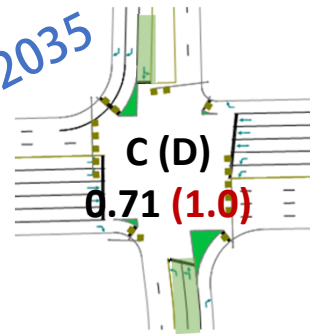
2040

Alternative 1
Full Intersection
Opening
(With Lane
Modifications)



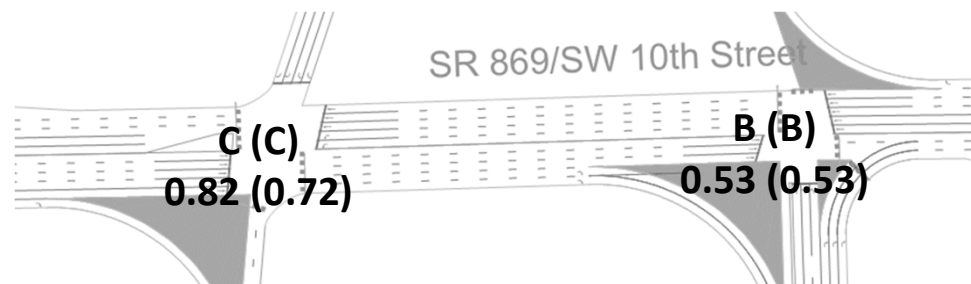
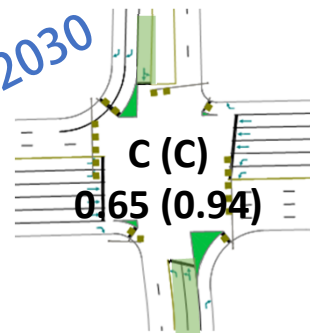
2035

Alternative 1
Full Intersection
Opening
(With Lane
Modifications)



2030

Alternative 1
Full Intersection
Opening
(With Lane
Modifications)



Legend & Notes

= volume exceeds capacity, queue may be longer
m = metered by upstream signal

New movement permitted (compared to the RCUT)

2035 AM Volume (veh/hr)

96	10	51	419
←	↓	↘	↖
1,768			431
SW 10 th St			

348	74	10	123
↖	↗	↘	↖
1,688			
536			
Newport Center Dr			

2035 PM Volume (veh/hr)

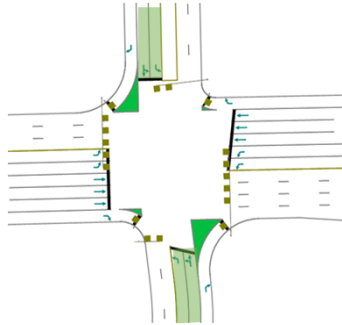
496	5	136	110
←	↓	↘	↖
1,755			119
SW 10 th St			

88	406	14	455
↖	↗	↘	↖
1,766			
79			
Newport Center Dr			

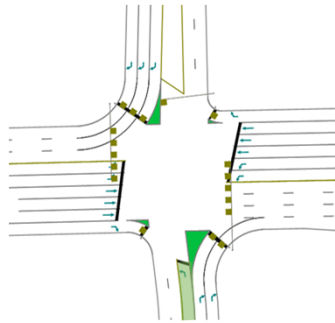
SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis

Overall Findings

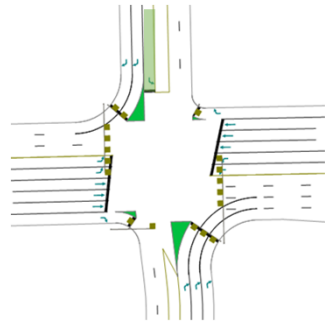
Alternative 1
Full Intersection
Opening
(With Lane
Modifications)



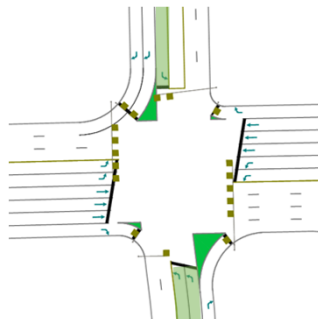
Alternative 2
Allow NBL



Alternative 3
Allow SBL



Alternative 4
NBL & SBL
Allowed,
but NBT & SBT
Prohibited

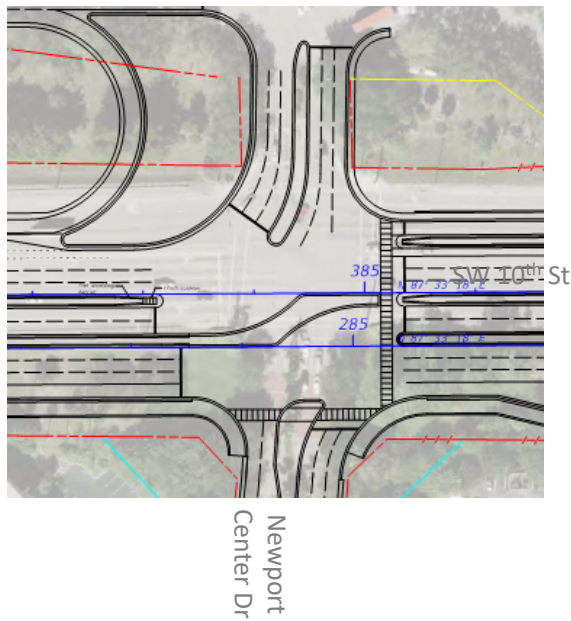


- At the I-95 Interchange:
 - Operations remain fairly consistent between the alternatives analysis in Synchro
- At Newport Center Drive:
 - Alternative 1, the full intersection opening with modifications to the existing striping, could have a service life through 2030/2035.
 - Alternative 2 that allows a NBL may be operationally feasible, but will preclude pedestrian crossings through the center median.
 - Alternative 3 that allows a SBL is not operationally sufficient ($V/C > 1$ and EBT queue nears storage).
 - Alternative 4 that prohibits the NBT and SBT is operationally promising, but designing the intersection to prohibit those movements needs further consideration.

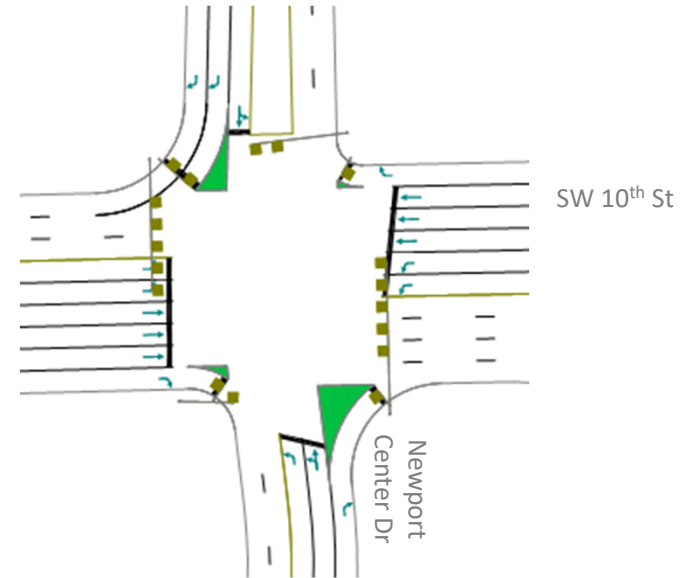
SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Recommendations

To meet local stakeholder expectations, evaluate Alternative 1 or Alternative 4 as an alternative to the RCUT.

PD&E Design

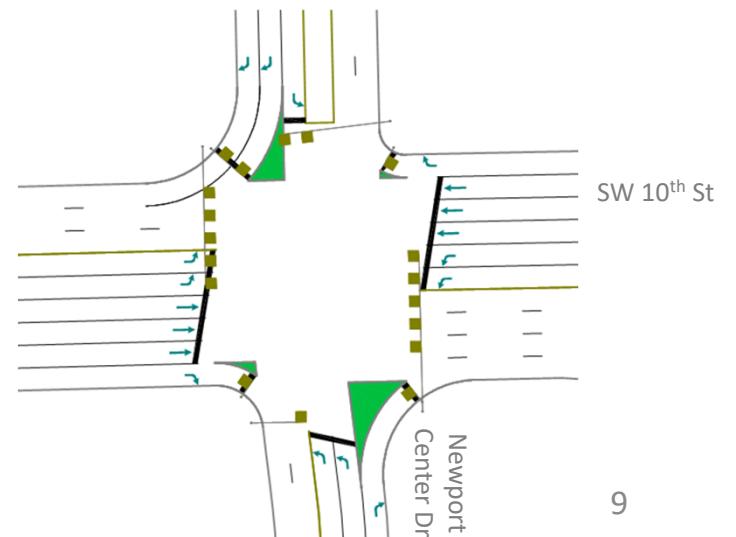


Alternative 1
Full Intersection Opening
(With Lane Modifications)

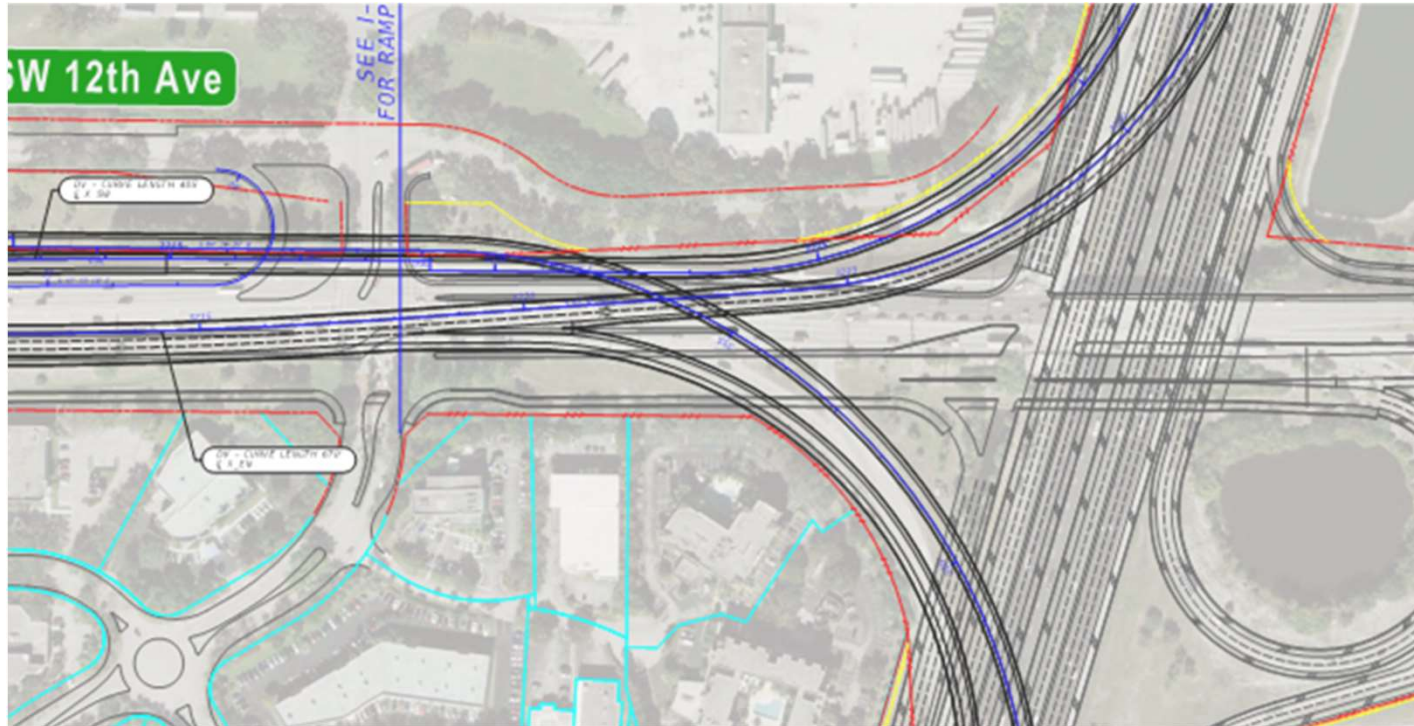


OR

Alternative 4
NBL & SBL Allowed,
but NBT & SBT Prohibited



January 20th, 2021



Agenda

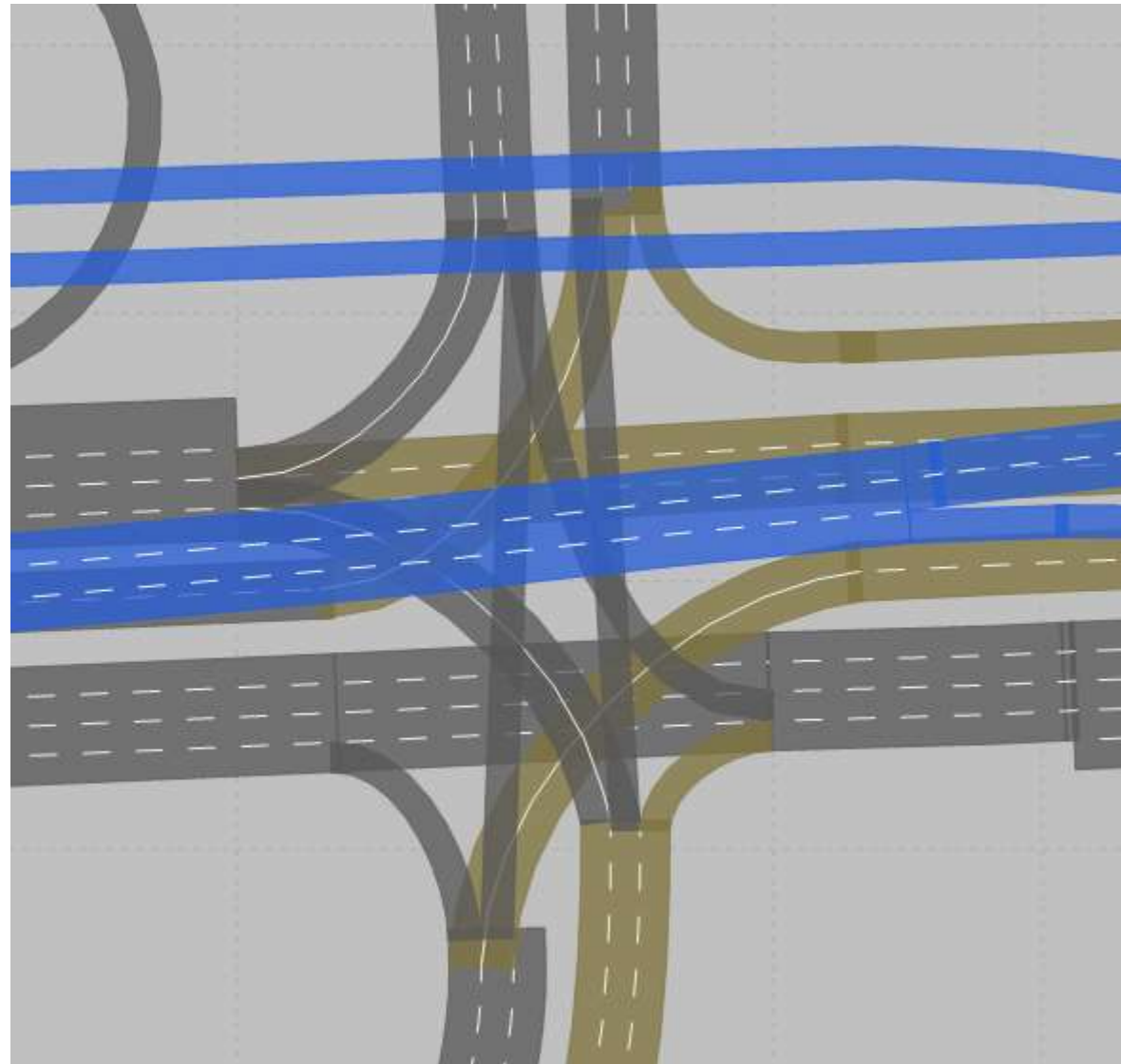
- Analysis Results
 - Intersection Queue Lengths
 - Intersection LOS
 - Traffic flow between Newport Center Drive, the Connector Lanes, and the System Interchange
- Recommendations and Discussion

Analysis Approach

- Modeled 2035 and 2040 AM and PM in Vissim 8
- Overall analysis includes: Newport Center Drive, I-95 West Ramp Terminal, I-95 East Ramp Terminal
- Corridor signals used timing from Synchro, adjusted splits as needed

SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Lane Configurations

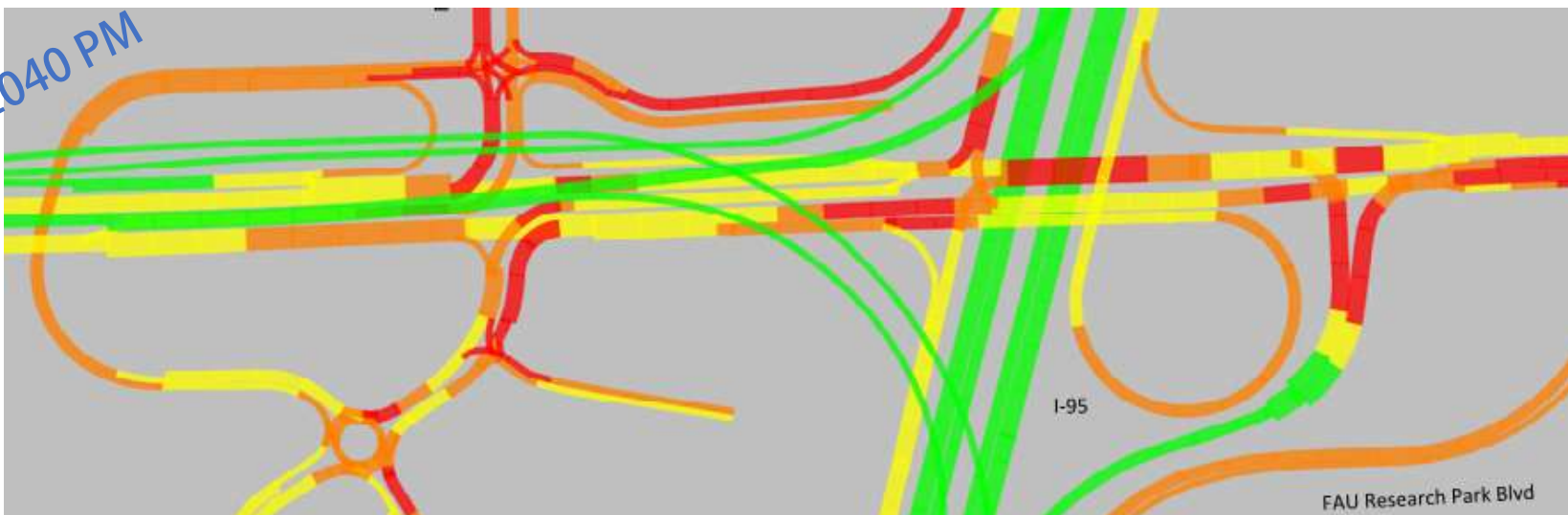
Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)



SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis

2040 PM — Average Travel Speed Comparison

PD&E Design – RCUT



Legend & Notes

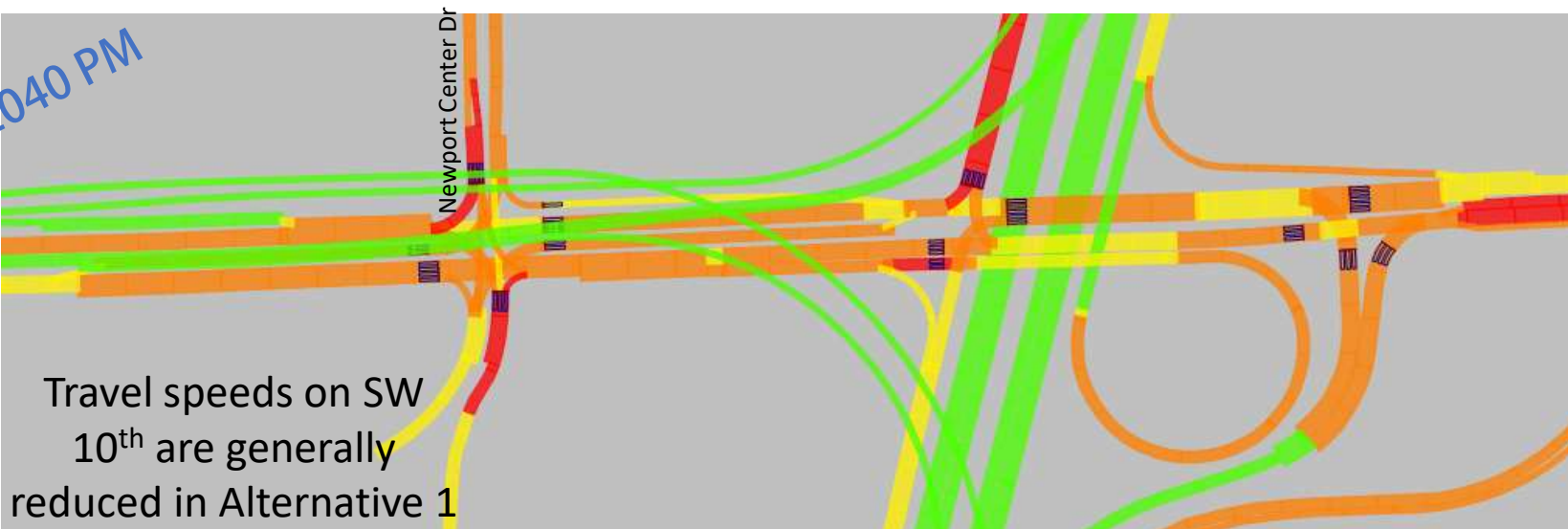
Legend

- < 15 mph
- 15-30 mph
- 30-45 mph
- > 45 mph

2040 PM Volume (veh/hr)

545 ←	5 ↓	150 ↘		120 ←	1,815 ↑	125 ↗
				SW 10 th St		
95 ←	1,870 →	85 ↓		435 ←	15 ↑	470 ↗
				Newport Center Dr		

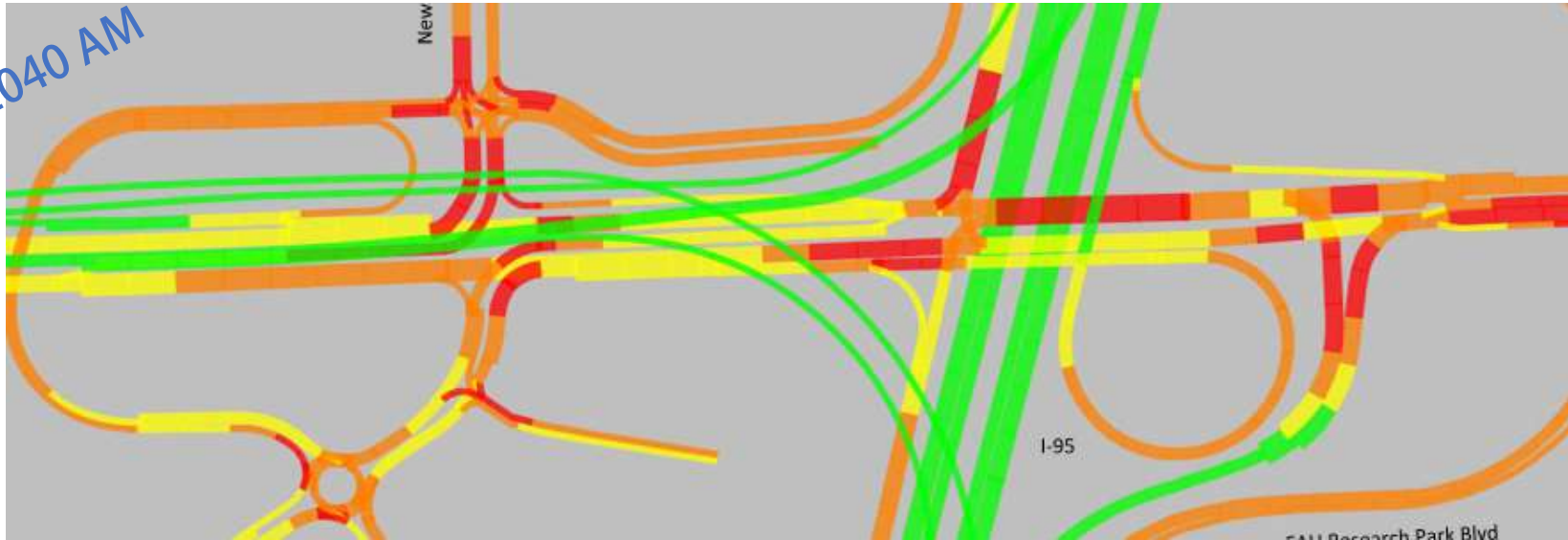
Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)



SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis

2040 AM — Average Travel Speed Comparison

PD&E Design – RCUT



Legend & Notes

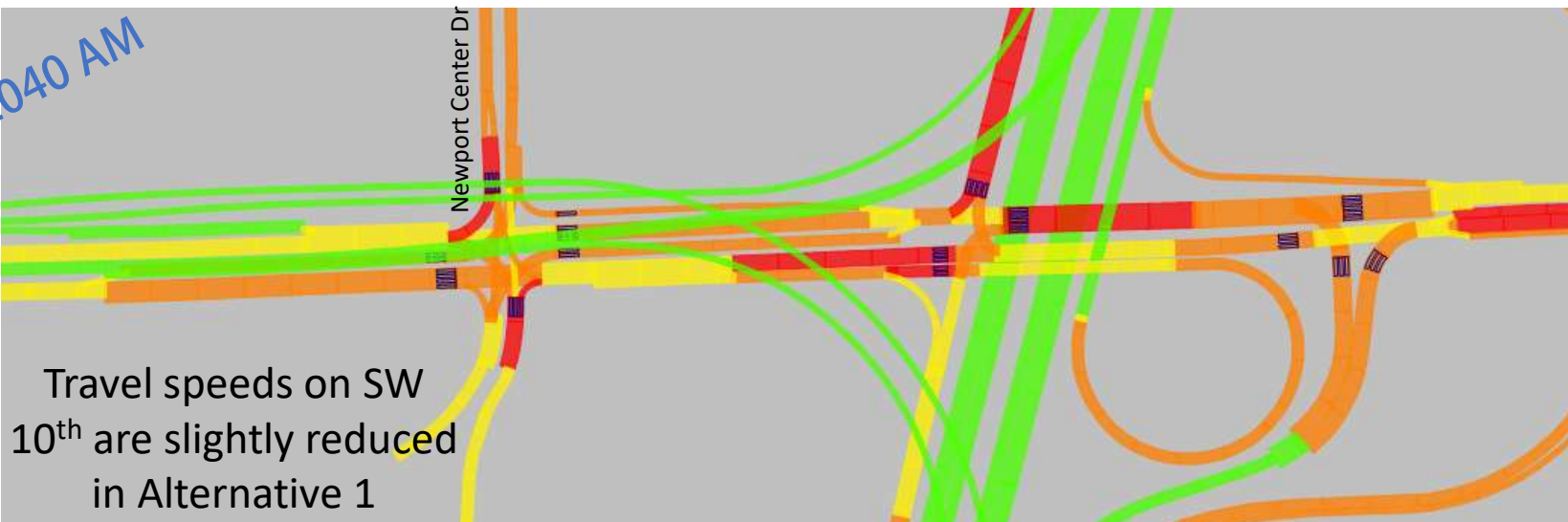
Legend

- < 15 mph
- 15-30 mph
- 30-45 mph
- > 45 mph

2040 AM Volume (veh/hr)

100	10	55	↖	435
↖	↓	↘	↖	1,885
			↘	445
				SW 10 th St
365	↖	↖	↖	↖
1,780	↖	↖	↖	80
550	↖	↖	↖	10
			↖	130
				Newport Center Dr

Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)



SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Traffic Snapshot of Alternative 1

Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)

2040 PM

Note: Roadway colors represent different types of roads. Color display can be adjusted as needed.



Screenshot taken during the peak hour, upon the start of the eastbound and westbound signal phase



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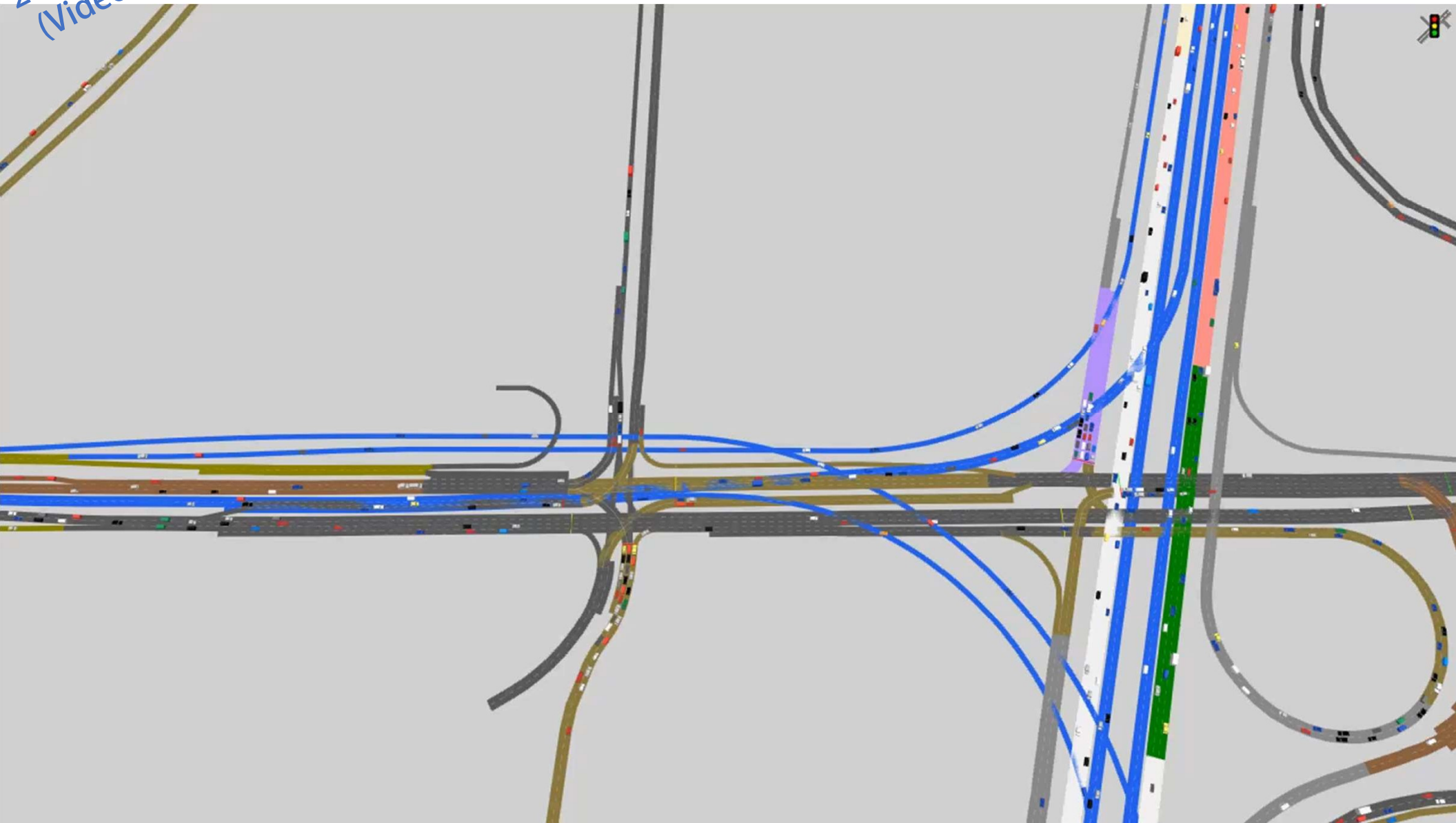
SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Traffic Simulation of Alternative 1

January 2021

Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)

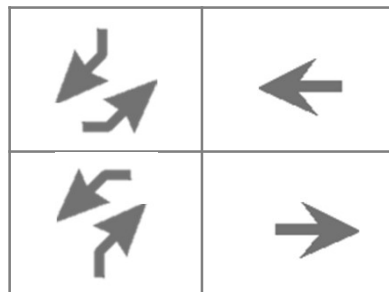
2040 PM
(Video)

Note: Roadway colors represent different types of roads. Color display can be adjusted as needed.



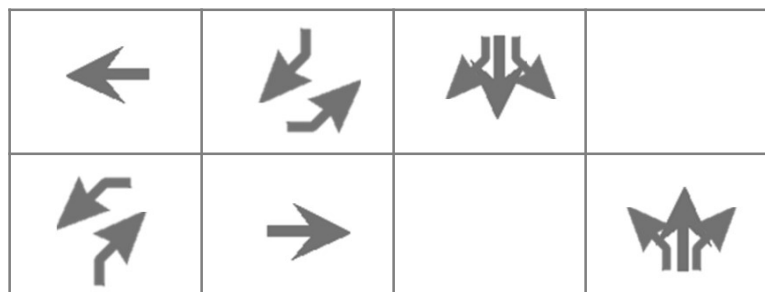
SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Signal Phasing/Timing

PD&E Design – RCUT



2-Phase Signal

Alternative 1 – Full Intersection Opening (With Lane Modifications Relative to Existing Condition)



4-Phase Signal



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SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis PM Vissim Results

January 2021

2040 PM
vs. 2040 PM

Intersection	Approach	Movement	2040 PTAR (Newport as RCUT)					2040 Alternative 1 (Newport as Full Intersection)				
			Volume	Max Queue	Delay and LOS (s/veh)			Volume	Max Queue	Delay and LOS (s/veh)		
					Movement	Approach	Int			Movement	Approach	Int
Newport Center Dr	EB	L	92	164	55 (D)	6.9 (A)	15.2 (B)	91	109	73.4 (E)	34.5 (C)	27.5 (C)
		T	1,837	613	5 (A)			1,846	617	32.5 (C)		
		R	83	613	5 (A)			82	617	36.4 (D)		
	WB	L	125	66	19 (B)	4.5 (A)		125	138	82.1 (F)	19.5 (B)	
		T	1,775	286	4 (A)			1,794	597	16.3 (B)		
		R	115	0	2 (A)			117	0	3 (A)		
	NB	L	591	322	49 (D)	49.4 (D)		454	338	48.7 (D)	32.8 (C)	
		T						18	246	45 (D)		
		R						489	355	17.6 (B)		
	SB	L	740	256	40 (D)	39.7 (D)		137	292	42.5 (D)	22.6 (C)	
		T						5	292	56.4 (E)		
		R						529	292	17.2 (B)		
I-95 Ramps at SW 10th Street	EB	L	827	620	25 (C)	26.3 (C)	34 (C)	840	385	12.3 (B)	18 (B)	
		T	895	400	45 (D)			919	370	34.6 (C)		
		R	699	0	3 (A)			708	8	3.1 (A)		
	WB	L	752	178	50 (D)	24.3 (C)		755	215	40.8 (D)	27.2 (C)	
		T	931	178	10 (A)			928	215	24.6 (C)		
		R	305	34	5 (A)			297	0	1.1 (A)		
	NB	L	520	232	48 (D)	50.1 (D)		524	223	47.6 (D)	47.6 (D)	
		R	691	294	52 (D)			676	276	47.7 (D)		
	SB	L	295	233	111 (F)	55 (D)		281	391	62.1 (E)	61.4 (E)	
		R	572	233	26 (C)			583	391	61 (E)		

Findings

WBT Max Queue increases but remains within the storage distance (~900 feet)

Exit ramp queues and delays remain similar to the RCUT

LOS F

Key movement for result comparison

Notes: (1) Signal timing and cycle lengths differ between the RCUT and the Full Intersection; the I-95 ramp terminal delay is slightly improved due to the signal timing differences. (2) The volume column confirms that the expected demand is reaching the intersections/interchanges.



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SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis PM Vissim Results

January 2021

2040 PM
vs. 2035 PM

			2040 PTAR (Newport as RCUT)					2035 Alternative 1 (Newport as Full Intersection)				
Intersection	Approach	Movement	Volume	Max Queue	Delay and LOS (s/veh)			Volume	Max Queue	Delay and LOS (s/veh)		
					Movement	Approach	Int			Movement	Approach	Intersecti on
Newport Center Dr	EB	L	92	164	55 (D)	6.9 (A)	15.2 (B)	87	116	70.7 (E)	29.1 (C)	24.6 (C)
		T	1,837	613	5 (A)			1,693	586	27 (C)		
		R	83	613	5 (A)			79	586	29.6 (C)		
	WB	L	125	66	19 (B)	4.5 (A)		113	114	81.5 (F)	17.6 (B)	
		T	1,775	286	4 (A)			1,651	570	14.2 (B)		
		R	115	0	2 (A)			114	0	2.9 (A)		
	NB	L	591	322	49 (D)	49.4 (D)		396	335	50.4 (D)	32.9 (C)	
		T						16	242	42.2 (D)		
		R						408	351	15.5 (B)		
	SB	L	740	256	40 (D)	39.7 (D)		124	262	48.6 (D)	21.1 (C)	
		T						4	262	45.1 (D)		
		R						458	262	13.4 (B)		
I-95 Ramps at SW 10th Street	EB	L	827	620	25 (C)	26.3 (C)	34 (C)	782	640	13.7 (B)	18.4 (B)	
		T	895	400	45 (D)			814	373	34.5 (C)		
		R	699	0	3 (A)			643	25	3.8 (A)		
	WB	L	752	178	50 (D)	24.3 (C)		671	182	38.7 (D)	25.1 (C)	
		T	931	178	10 (A)			863	182	22.6 (C)		
		R	305	34	5 (A)			286	0	1.1 (A)		
	NB	L	520	232	48 (D)	50.1 (D)		495	197	42.2 (D)	42.9 (D)	
		R	691	294	52 (D)			607	244	43.5 (D)		
	SB	L	295	233	111 (F)	55 (D)		263	397	66 (E)	63 (E)	
		R	572	233	26 (C)			519	397	61.5 (E)		

LOS F

Key movement for result comparison

Notes: (1) Signal timing and cycle lengths differ between the RCUT and the Full Intersection; the I-95 ramp terminal delay is slightly improved due to the signal timing differences. (2) The volume column confirms that the expected demand is reaching the intersections/interchanges.



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SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis AM Vissim Results

January 2021

2040 AM
vs. 2040 AM

Findings

EBT and WBL delay increases, but max queue remains similar to the RCUT

SB exit ramp queues increase but remain near the storage distance (~625 feet before single lane section and additional 1,000+ feet after)

Intersection	Approach	Movement	2040 PTAR (Newport as RCUT)					2040 Alternative 1 (Newport as Full Intersection)				
			Volume	Max Queue	Delay and LOS (s/veh)			Volume	Max Queue	Delay and LOS (s/veh)		
					Movement	Approach	Int			Movement	Approach	Int
Newport Center Dr	EB	L	359	315	84 (F)	16.9 (B)	14.4 (B)	345	281	66.2 (E)	27.8 (C)	23.7 (C)
		T	1,776	640	6 (A)			1,717	640	20.5 (C)		
		R	545	640	9 (A)			504	640	26.1 (C)		
	WB	L	431	208	28 (C)	9.1 (A)	14.4 (B)	431	292	73.4 (E)	19 (B)	
		T	1,852	254	6 (A)			1,867	313	9.9 (A)		
		R	428	60	4 (A)			428	0	3.8 (A)		
	NB	L	178	107	41 (D)	40.7 (D)	14.4 (B)	88	105	68.2 (E)	29.5 (C)	
		T						11	10	37.9 (D)		
		R						135	122	3.6 (A)		
	SB	L	140	112	35 (C)	35.3 (D)	14.4 (B)	55	158	65.6 (E)	31.2 (C)	
		T						12	158	68.9 (E)		
		R						89	158	4.8 (A)		
I-95 Ramps at SW 10th Street	EB	L	532	369	19 (B)	30 (C)	43.6 (D)	518	408	15.1 (B)	26.4 (C)	
		T	886	354	54 (D)			867	379	47.8 (D)		
		R	537	0	1 (A)			517	0	1.9 (A)		
	WB	L	766	392	103 (F)	47.3 (D)	43.6 (D)	773	197	43.6 (D)	33.6 (C)	
		T	1,109	392	21 (C)			1,108	197	34.7 (C)		
		R	282	142	1 (A)			274	0	1 (A)		
	NB	L	768	364	58 (E)	55.9 (E)	43.6 (D)	770	298	53.8 (D)	52 (D)	
		R	479	228	52 (D)			465	212	49.1 (D)		
	SB	L	409	333	72 (E)	46 (D)		43.6 (D)	396	648	51.4 (D)	53.5 (D)
		R	835	333	33 (C)		847		648	54.5 (D)		

LOS F

Key movement for result comparison

Notes: (1) Signal timing and cycle lengths differ between the RCUT and the Full Intersection; the I-95 ramp terminal delay is slightly improved due to the signal timing differences. (2) The volume column confirms that the expected demand is reaching the intersections/interchanges.



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SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis AM Vissim Results

January 2021

2040 AM
vs. 2035 AM

			2040 PTAR (Newport as RCUT)					2035 Alternative 1 (Newport as Full Intersection)				
Intersection	Approach	Movement	Volume	Max Queue	Delay and LOS (s/veh)			Volume	Max Queue	Delay and LOS (s/veh)		
					Movement	Approach	Int			Movement	Approach	Int
Newport Center Dr	EB	L	359	315	84 (F)	16.9 (B)	14.4 (B)	342	274	66.5 (E)	27.9 (C)	23.8 (C)
		T	1,776	640	6 (A)			1,668	650	20.4 (C)		
		R	545	640	9 (A)			494	650	26.3 (C)		
	WB	L	431	208	28 (C)	9.1 (A)		415	294	74.4 (E)	19 (B)	
		T	1,852	254	6 (A)			1,789	302	9.7 (A)		
		R	428	60	4 (A)			406	0	3.4 (A)		
	NB	L	178	107	41 (D)	40.7 (D)		82	103	68.8 (E)	29.6 (C)	
		T						11	10	43.6 (D)		
		R						127	120	3.1 (A)		
	SB	L	140	112	35 (C)	35.3 (D)		52	161	65.3 (E)	31.1 (C)	
		T						11	161	68.6 (E)		
		R						84	162	5 (A)		
I-95 Ramps at SW 10th Street	EB	L	532	369	19 (B)	30 (C)	43.6 (D)	502	368	15.2 (B)	25.6 (C)	
		T	886	354	54 (D)			831	406	46.1 (D)		
		R	537	0	1 (A)			499	0	1.9 (A)		
	WB	L	766	392	103 (F)	47.3 (D)		735	184	42.3 (D)	32.5 (C)	
		T	1,109	392	21 (C)			1,056	184	33.4 (C)		
		R	282	142	1 (A)			261	15	1 (A)		
	NB	L	768	364	58 (E)	55.9 (E)		729	292	52 (D)	51 (D)	
		R	479	228	52 (D)			450	212	49.5 (D)		
	SB	L	409	333	72 (E)	46 (D)		384	534	49.4 (D)	53 (D)	
		R	835	333	33 (C)			823	534	54.6 (D)		

LOS F

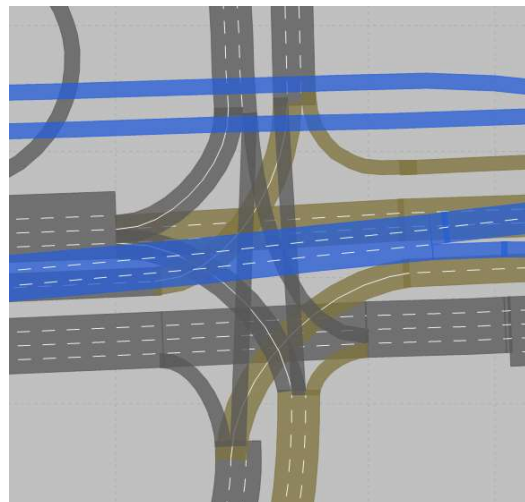
Key movement for result comparison

Notes: (1) Signal timing and cycle lengths differ between the RCUT and the Full Intersection; the I-95 ramp terminal delay is slightly improved due to the signal timing differences. (2) The volume column confirms that the expected demand is reaching the intersections/interchanges.

SW 10th Street CDC - Newport Center Drive Intersection Alternatives Analysis Overall Findings

1. The eastbound and westbound approaches at Newport Center Drive can still accommodate the 2040 traffic volumes in the “full intersection” configuration. **However**, the northbound and southbound approaches may need to wait more than 1 signal cycle to exit the intersection.
2. Travel speeds on SW 10th are reduced during the peak hour but max queues are not anticipated to exceed storage.
3. Compared to the RCUT configuration, the operations at the I-95 ramp terminals remain fairly consistent for the “full intersection” configuration.
4. While pedestrian phases are not modeled, any minor street pedestrian call is expected to significantly degrade operations on SW 10th Street.

Based on model assumptions and results, the full intersection opening at Newport Center Drive can be implemented through approximately 2035 or 2040.



MEMORANDUM

Date: August 10, 2018

To: Robert Bostian, P.E., FDOT D4
Hui Zhao, P.E., FDOT D4

From: Pramod Choudhary, P.E., PTOE, AECOM
Andrew Velasquez, P.E., PTOE, AECOM/Florida's Turnpike

Copies: Anson Sonnet, P.E., FDOT D4
Vilma Croft, P.E., HNTB
Lisa Dykstra, P.E., RS&H

Subject: SW 10th Street at I-95 – Alternatives Analysis Memorandum

FPID(s): 436964-1 and 439891-1

State Road: SW 10th Street (S.R. 869)

County: Broward

DRAFT

INTRODUCTION

The purpose of this memorandum is to document the various alternatives that were developed and analyzed for the I-95 and SW 10th Street PD&E Study (FPID 436964-1) and the SW 10th Street Connector PD&E Study (FPID 439891-1). Traffic evaluation was conducted for two different Build Alternatives (a depressed Center Alignment and a depressed Northern Alignment for the proposed managed lanes), with six different managed lanes ingress and egress configurations resulting in a total of twelve (12) Build Alternatives. In addition, 2016 Existing Conditions, 2040 No-Build Conditions, and 2040 Partial Build Conditions were also evaluated.

The fifteen scenarios (existing 2016 conditions, 2040 No-Build conditions, 2040 Partial Build conditions, and twelve 2040 Build Concept conditions) were first analyzed by conducting a Tier 1 volume to capacity ratio analysis of the SW 10th Street local lanes and proposed managed lanes. In addition, the vehicle-miles traveled in the managed lanes during the peak hours for each of the twelve Build Concepts were calculated and compared. Subsequently, a Tier 2 intersection operations analysis was completed for the signalized intersections along SW 10th Street. A Tier 2 freeway analysis of the proposed managed lanes connecting the Sawgrass, Florida's Turnpike and I-95 was also completed. The peak hour traffic operations analysis results were reviewed to screen the twelve Build Concepts for any traffic operations fatal flaws, and the comparison of results was used to identify the most advantageous Build Concepts to be considered further.

Overall, Tier 1 and Tier 2 analyses resulted in the selection of the North Build Alternative 3D-1.3 and the Center Build Alternative 3D-1.3 as the top ranked alternatives. Please refer to the *Traffic Analysis Technical Memorandum* dated May 4, 2018 and prepared by RS&H. VISSIM microsimulation was conducted to further evaluate these two shortlisted alternatives. The North Build Alternative 3D-1.3 was found to provide better operating conditions than the Center Build Alternative. Hence, the North Build Alternative 3D-1.3 was operationally considered as the best Build Alternative and was further refined to improve the overall operations. One of the refinements was to modify the Newport Drive intersection to eliminate the northbound and southbound through and left-turn movements from the intersection and convert the unsignalized intersection of SW 12th Avenue and Newport Drive into a roundabout.

The documentation provided herein includes the lane geometry, traffic volumes, and intersection analysis for the No-Build, Partial Build and Build 3D-1.3 alternatives along SW 10th Street from Military Trail to FAU Research Boulevard. In addition, VISSIM traffic simulation results of the shortlisted alternatives have also been included to help in the determination of the operationally best alternative.

LANE GEOMETRY

Figures 1 through 3 provide the lane geometry for the future year alternatives described below:

No-Build Alternative - This alternative assumes future capacity with the Turnpike Mainline widening, 95 Express Phase 3 Lanes and a portion of the Sawgrass Expressway Widening from Sunrise Boulevard to U.S. 441. No improvements are included along the Sawgrass Expressway from U.S. 441 to Powerline Road and along SW 10th Street. The No-Build includes the intersection improvements under construction at the SW 10th Street/I-95 interchange and Hillsboro Boulevard/I-95 interchange.

Partial Build Alternative - In addition to the No-Build improvements, the Partial-Build alternative assumes:

- Full Interchange at Turnpike Mainline/Sawgrass Expressway/SW 10th Street.
- Direct Connections to northbound and southbound 95 Express.
- Modification to the I-95 interchange ramp terminals to include additional turn lanes and a new westbound to northbound ramp.

This alternative assumes that the 95 Express direct connect ramps will extend west of Military Trail via grade-separated ramps. The full interchange at Turnpike Mainline/Sawgrass Expressway will connect to an at-grade SW 10th Street arterial west of the Powerline Road intersection. SW 10th Street also remains as a four-lane arterial between Powerline Road and Military Trail.

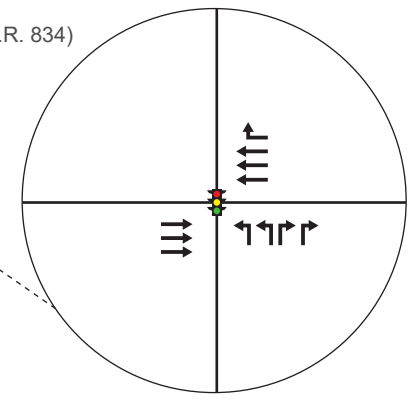
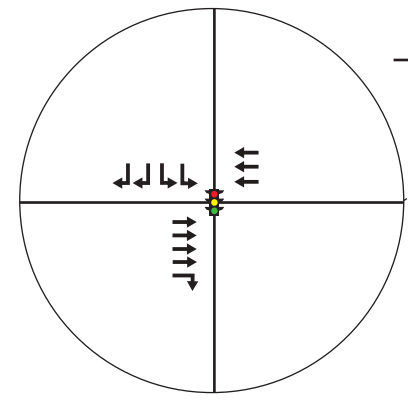
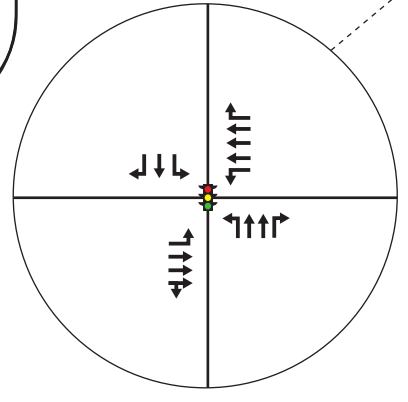
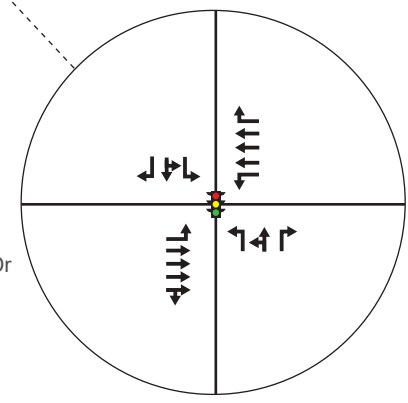
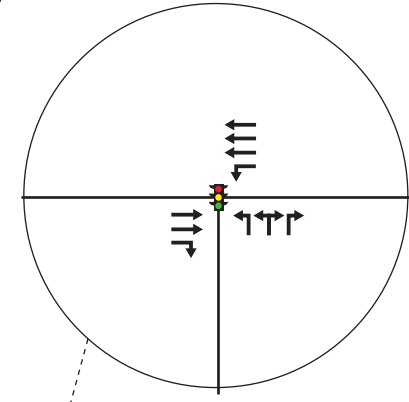
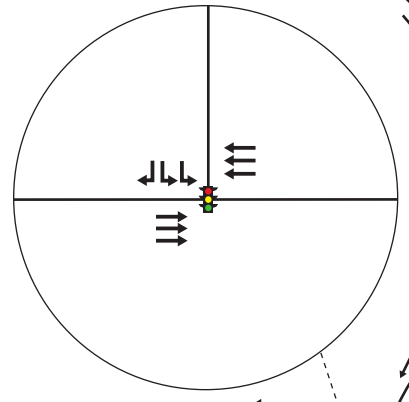
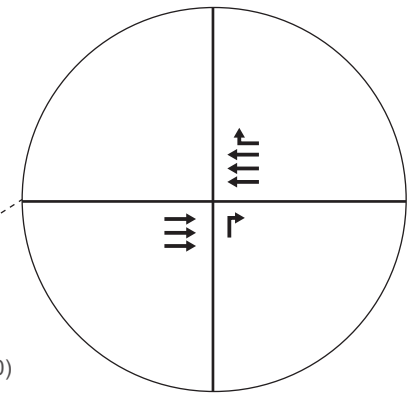
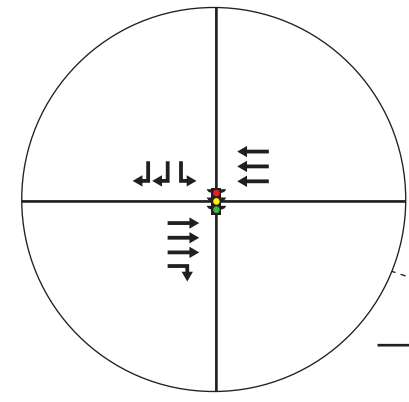
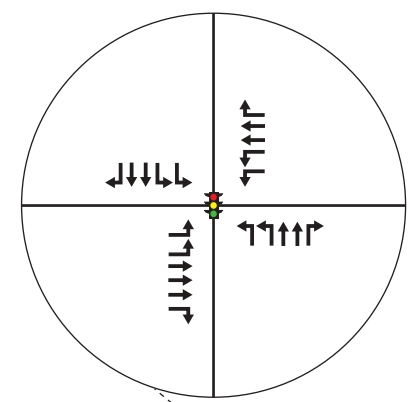
Build SW 10th Street Alternative 3D-1.3 North Alignment and Modified Newport Drive Intersection (North Modified) - In addition to the Partial-Build improvements, the Build alternative includes:

- Four managed lanes (2 in each direction) along SW 10th Street with grade separation at Powerline Road and Military Trail intersections.
- Managed lane ingress and egress ramps on either side of the Military Trail intersection.
- Removal of the northbound and southbound left turns at Newport Drive with additional northbound and southbound right turn lanes.
- Access from eastbound SW 12th Avenue to westbound SW 10th Street managed lanes.
- A Roundabout at the intersection of East Newport Center Drive and West Newport Center Drive.

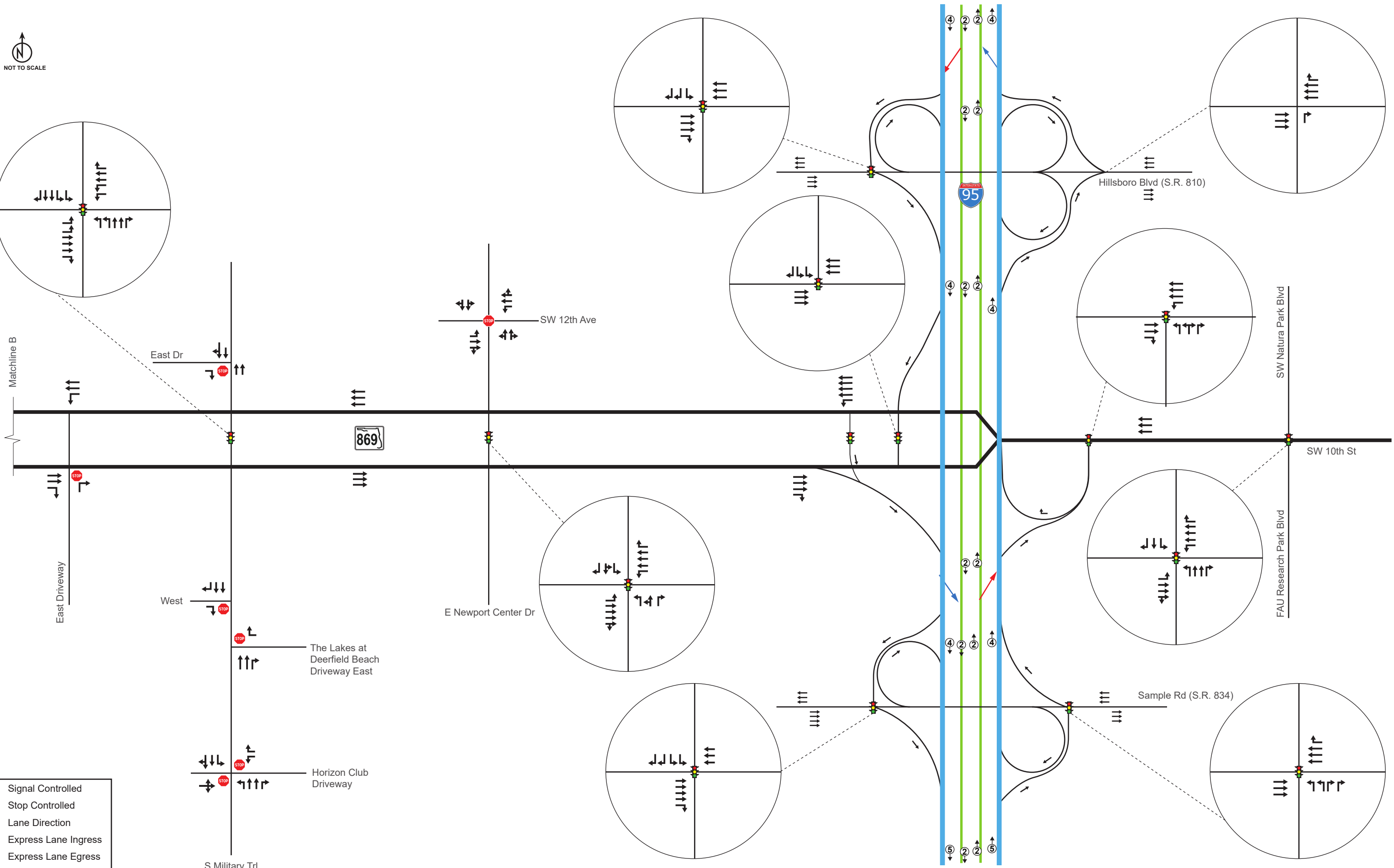
Through coordination with the SW 10th Street Connector PD&E team, 12 Build Alternatives (3D 1.1 through 1.6 Center and North Alignments) were developed to evaluate the best potential ingress/egress combination along SW 10th Street. Build Alternatives 3D-1.3 Center and 3D-1.3 North alignments reflected the highest ranked ingress/egress combination through the tiered screening process. For simplicity, in comparing the intersection analysis results, Alternative 3D 1.3 North alignment is compared against the Partial and No-Build alternatives.

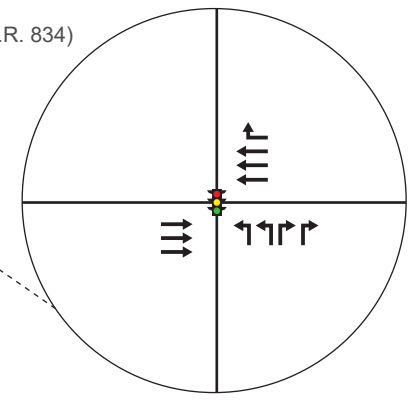
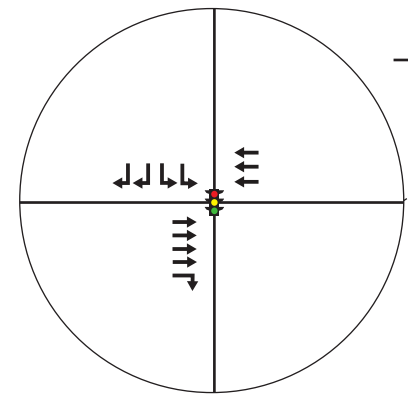
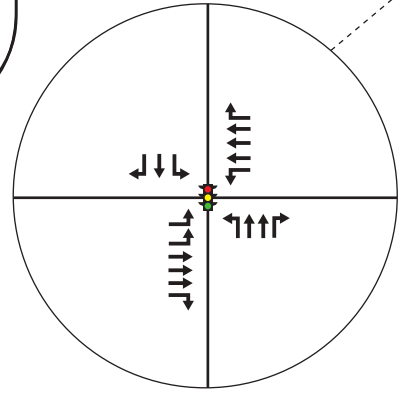
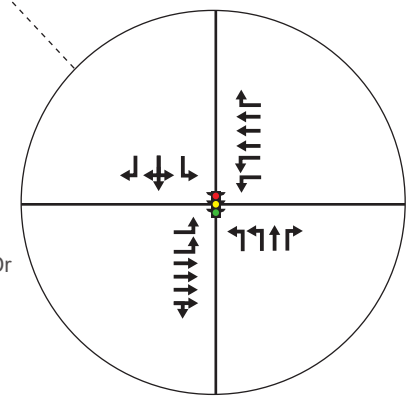
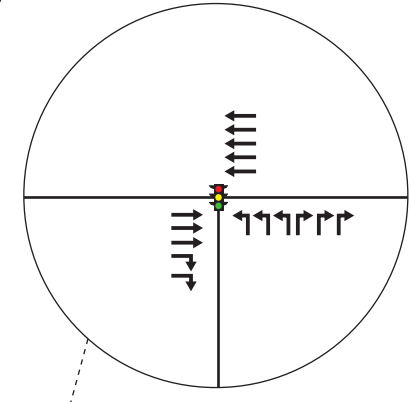
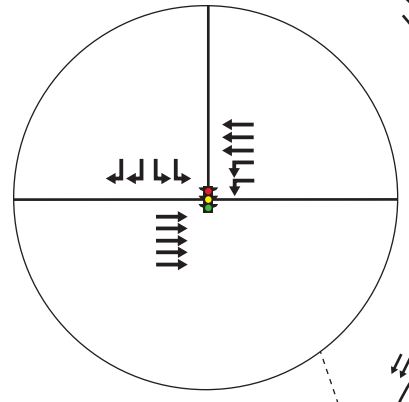
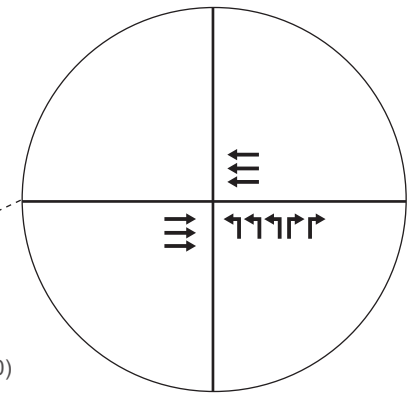
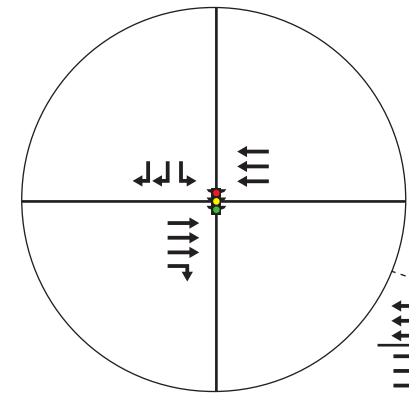
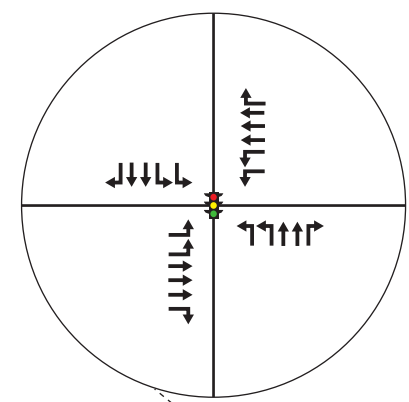
TRAFFIC FORECASTS

Figures 4 through 6 show the 2040 Directional Design Hour Volumes (DDHVs) for No-Build, Partial Build and Build 3D.1.3 alternatives. The No-Build and Partial Build alternatives were provided in the *Draft SW 10th Street Project Traffic Forecast Memorandum (PTFM)*, dated January 2018. The PTFM also included the forecasts for Build Alternative 3-D 1.1. The DDHVs for Build Alternative 3D-1.3 North Modified were developed in coordination with the SW 10th Street Connector PD&E team using output from the Express Lanes Time-of-Day Model and then manually reassigning traffic to accommodate median closures or restricted movements.

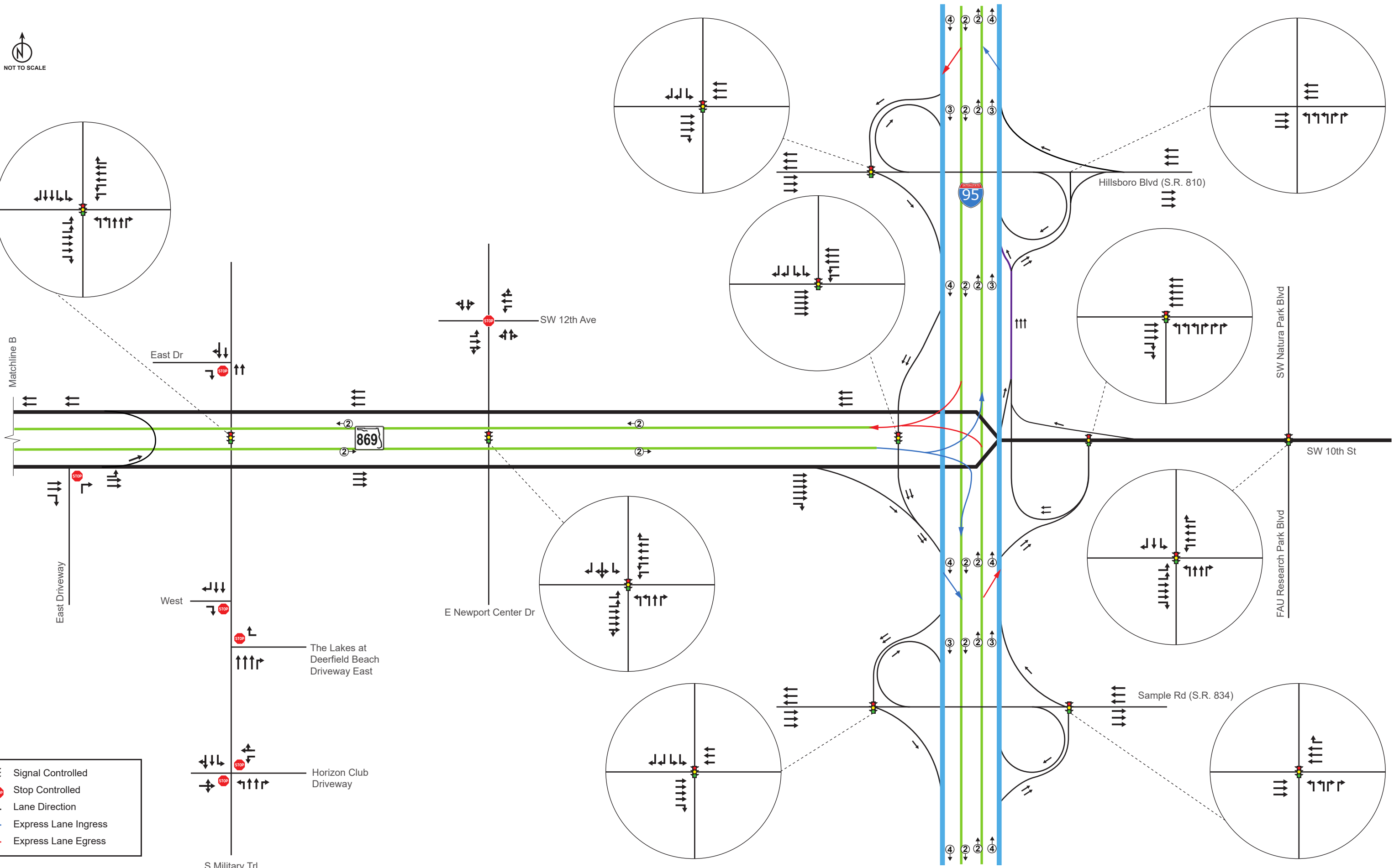


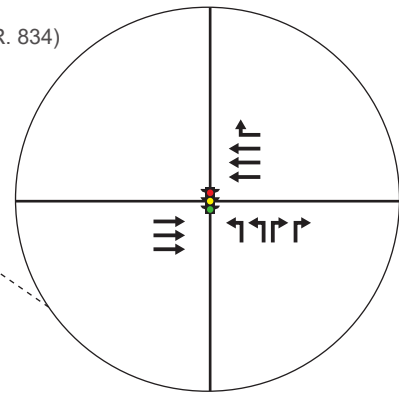
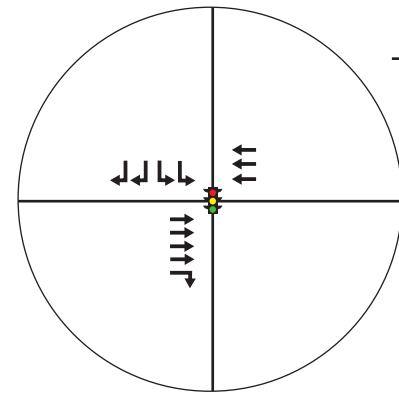
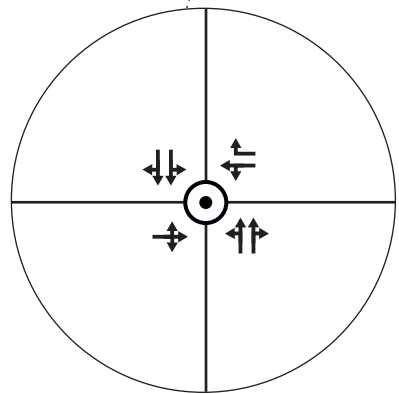
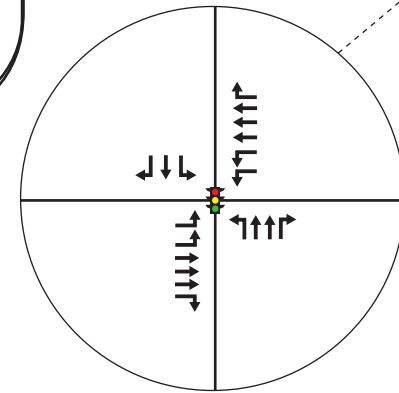
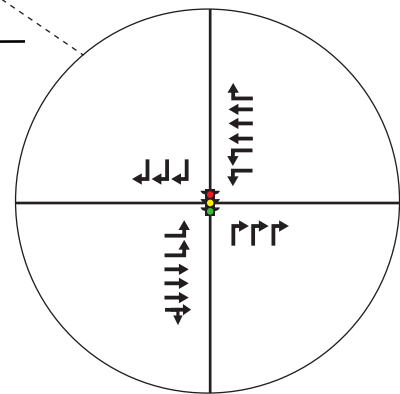
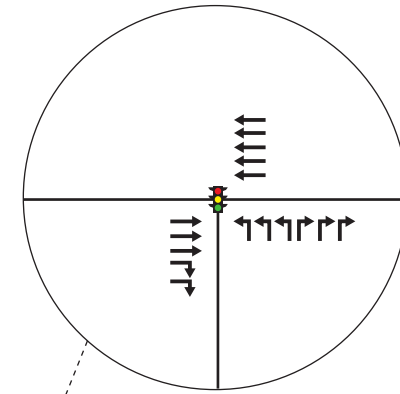
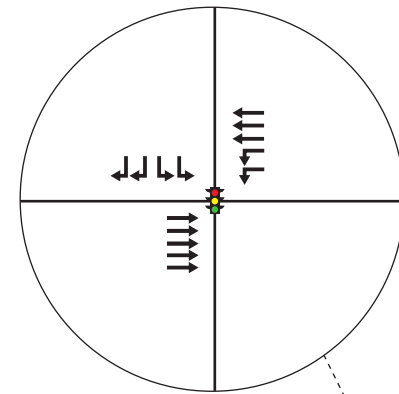
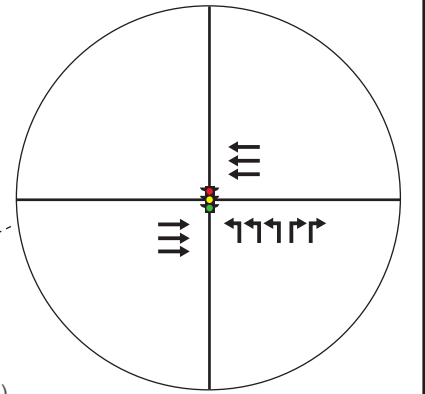
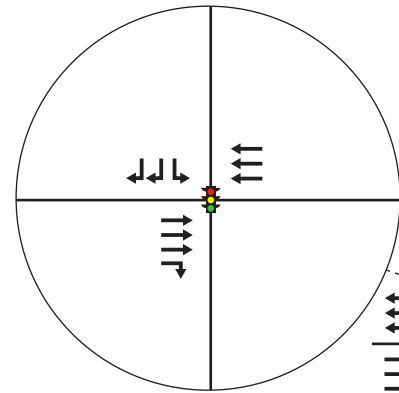
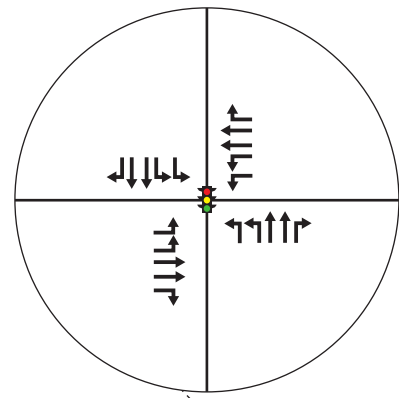
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- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress



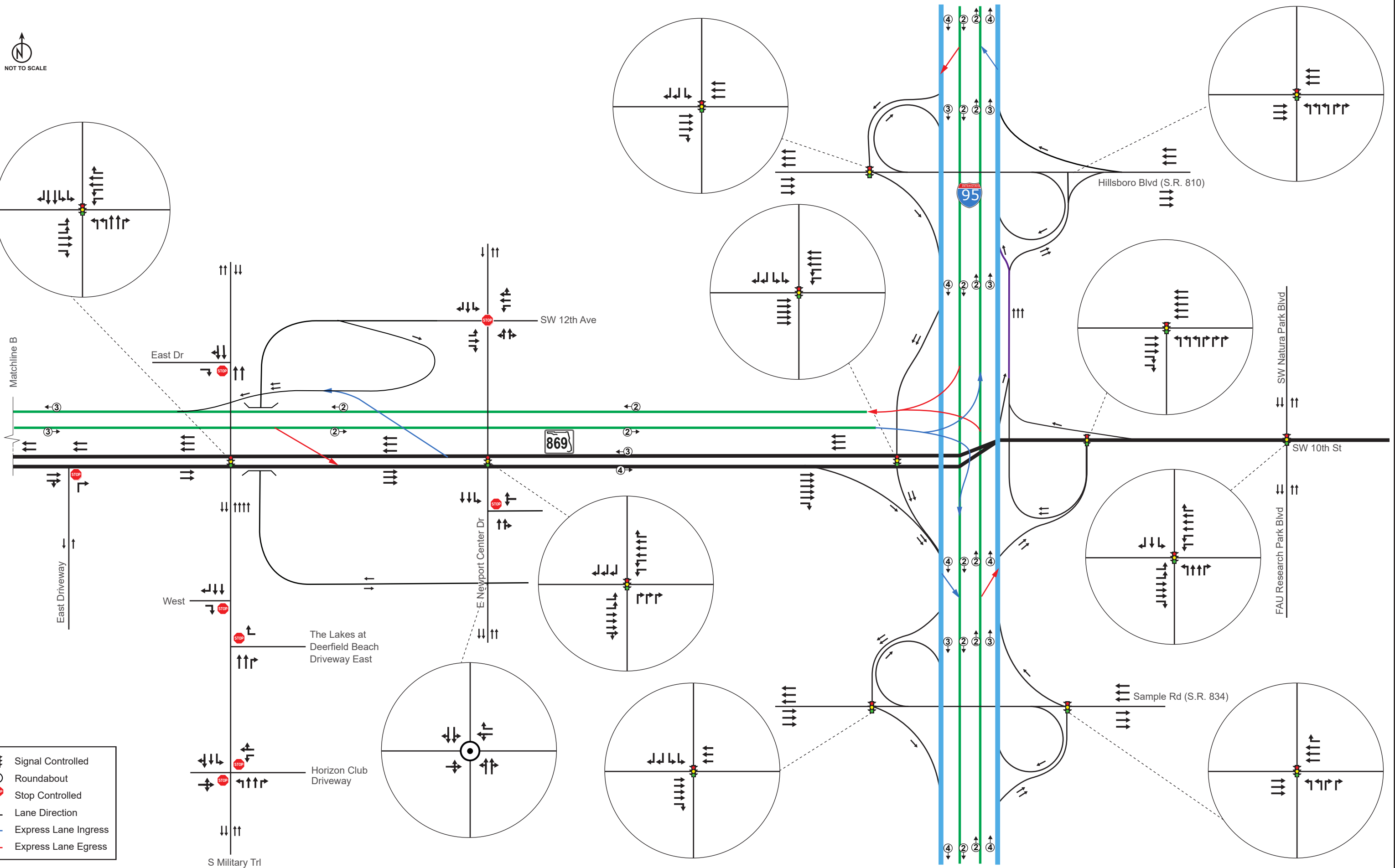


- Signal Controlled
- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress



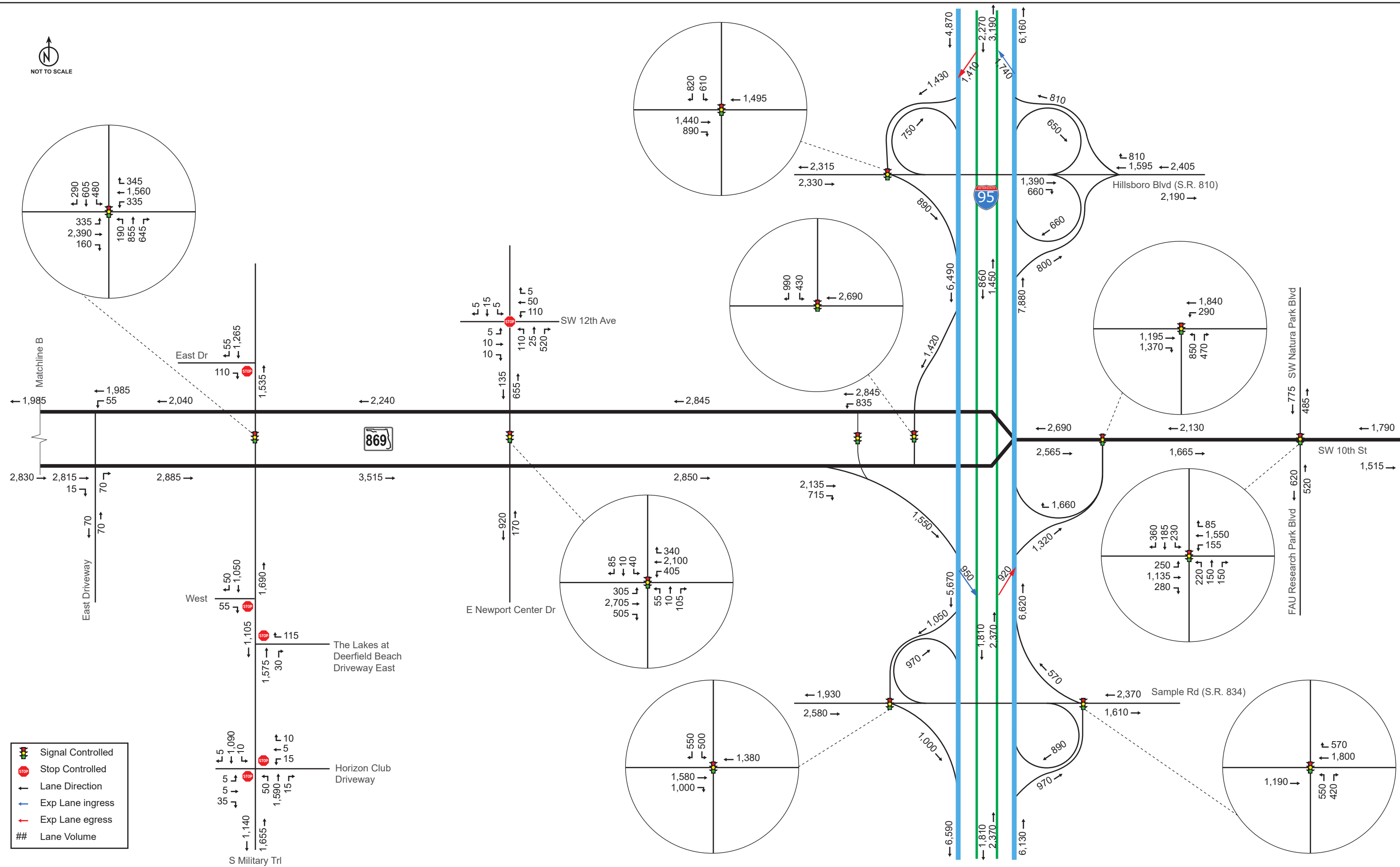


- Signal Controlled
- Roundabout
- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress



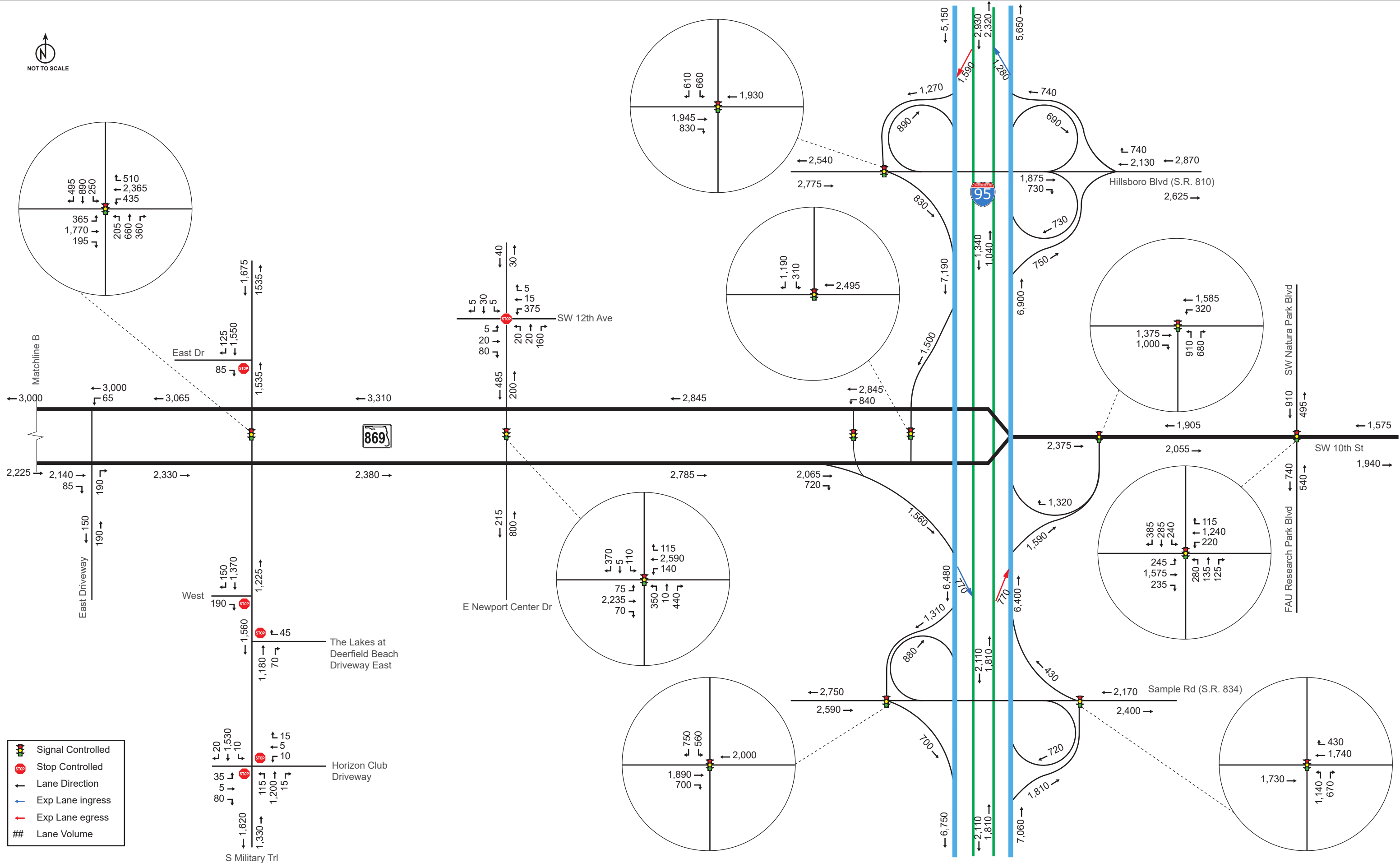


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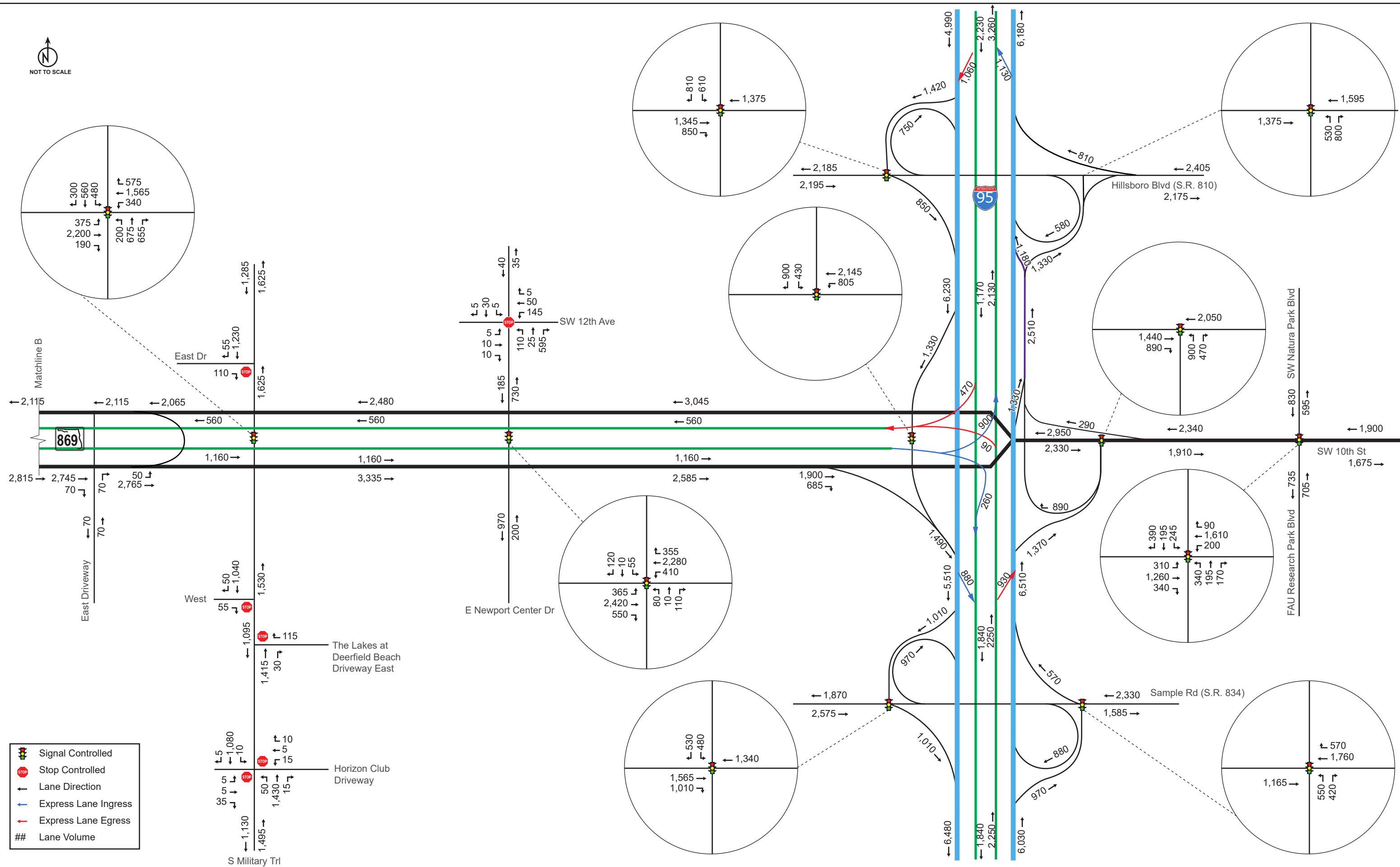


- Signal Controlled
- Stop Controlled
- Lane Direction
- Exp Lane ingress
- Exp Lane egress
- Lane Volume



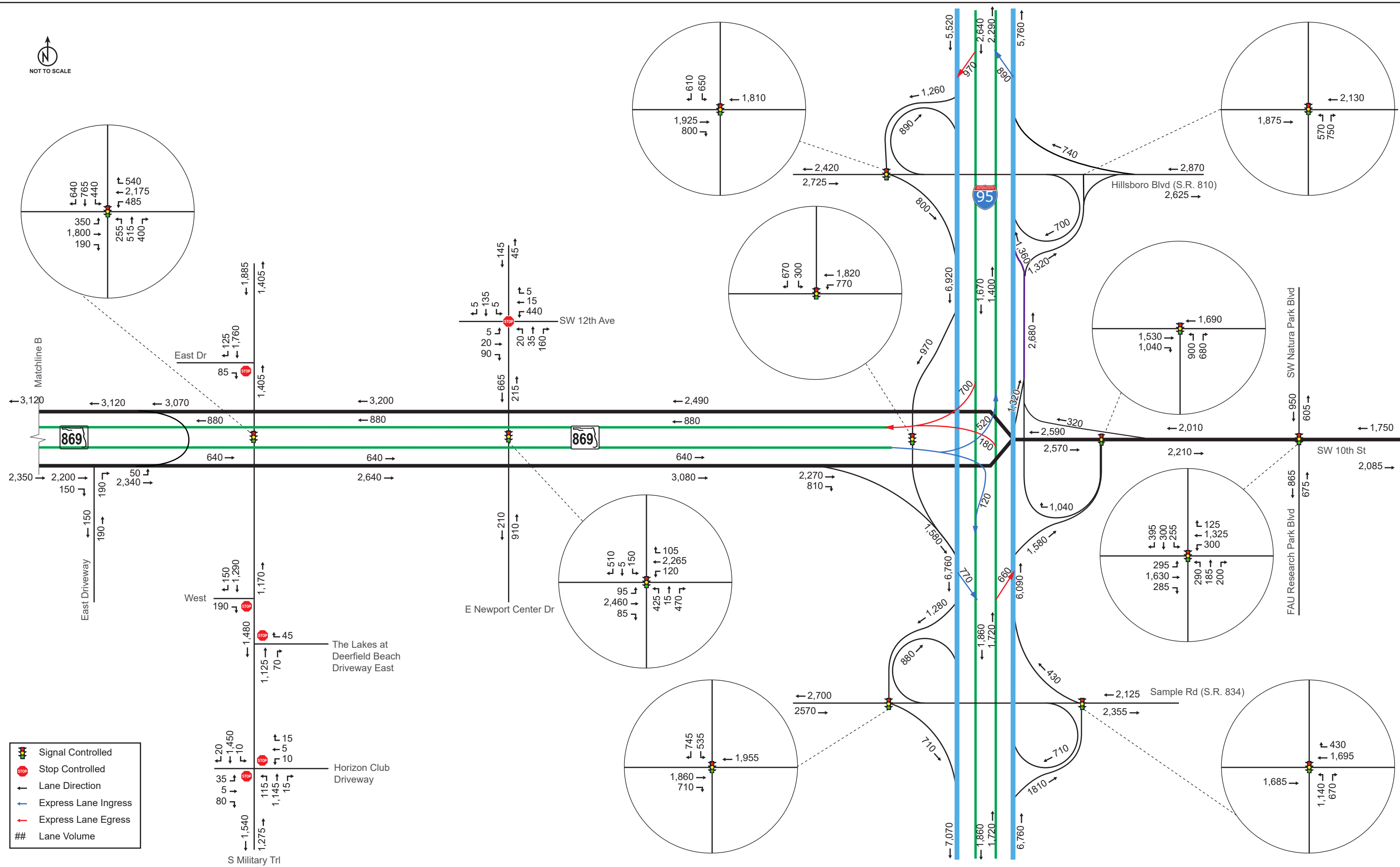


- Signal Controlled
- Stop Controlled
- Lane Direction
- Exp Lane ingress
- Exp Lane egress
- Lane Volume

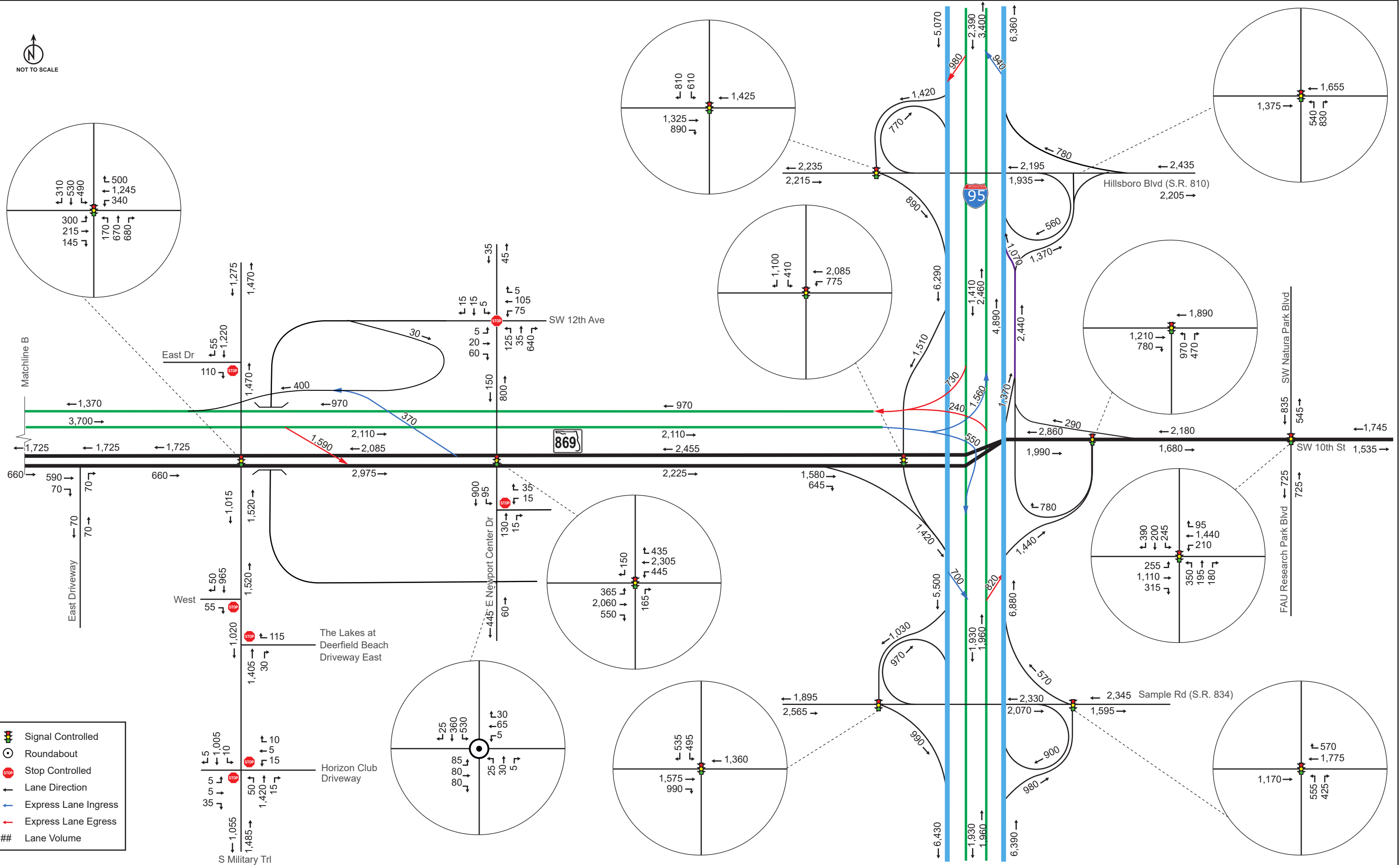


- Signal Controlled
- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress
- Lane Volume



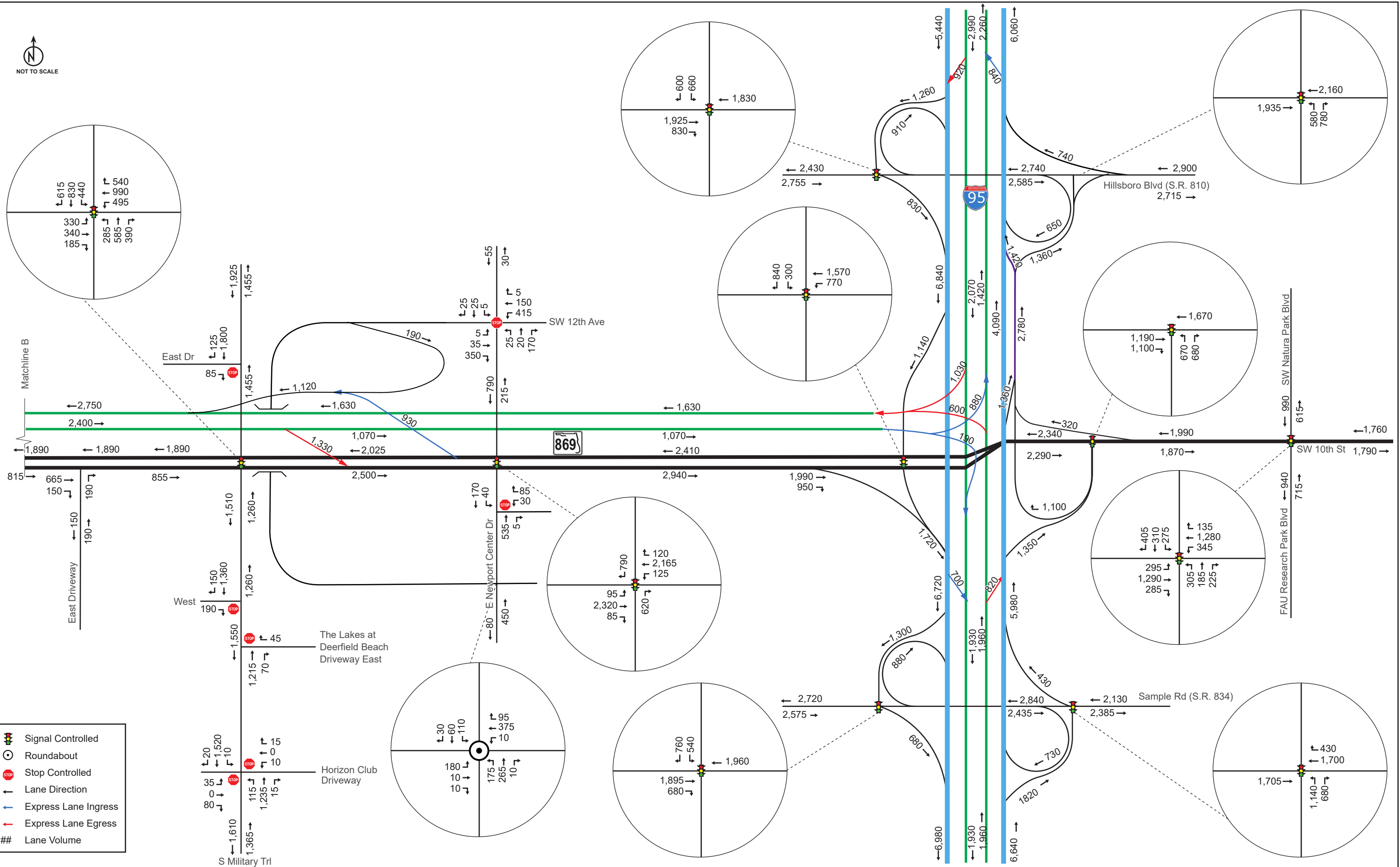


- Signal Controlled
- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress
- Lane Volume



- Signal Controlled
- Roundabout
- Stop Controlled
- Lane Direction
- Express Lane Ingress
- Express Lane Egress
- Lane Volume





INTERSECTION ANALYSIS RESULTS

Tables 1 through 3 present the 2040 level of service and control delays (in seconds/vehicle) for the No-Build, Partial Build, and Build 3D1.3 North Alignment (Modified), respectively, for the SW 10th Street intersections from Military Trail to FAU Research Boulevard. The analysis was conducted using Synchro software and reported as Highway Capacity Manual 2000 output, consistent with the SW 10th Street PTFM methodology. The results show the following:

2040 AM Peak Hour

1. The analysis generally shows a progressive improvement (reduction) in overall intersection delays from the No-Build to Partial Build to Build alternatives.
2. At the Military Trail intersection, the Partial Build alternative is expected to reduce the delay by more than 62 seconds/vehicle from the No-Build alternative and the Build alternative would further reduce it by another 34 seconds/vehicle. The Build alternative provides LOS D at this intersection.
3. The Newport Center Drive intersection is expected to operate at LOS D, C and B under the No-Build, Partial Build and Build alternatives, respectively.
4. The I-95 SB Ramp intersection is expected to operate at LOS E, D and D under the No-Build, Partial Build and Build alternatives, respectively. The average vehicle delays between the Partial Build and Build alternatives are comparable.
5. The I-95 NB Ramp intersection is expected to operate at LOS F, C and C under the No-Build, Partial Build and Build alternatives, respectively. The Partial Build and Build alternatives would reduce the overall intersection delays by more than 54 seconds per vehicle when compared with the No-Build alternative. The average vehicle delays between the Partial Build and Build alternatives are comparable.
6. The FAU Research Park Boulevard is expected to operate at LOS D under the No-Build, Partial Build and Build alternatives with comparable delays.

2040 PM Peak Hour

1. The analysis generally shows a progressive improvement (reduction) in overall intersection delays from the No-Build to Partial Build to Build alternatives.
2. At the Military Trail intersection, the Partial Build alternative would reduce the delay by 72 seconds/vehicle from the No-Build alternative and the Build alternative would further reduce it by another 31 seconds/vehicle. The intersection is expected to operate at LOS F, F and D under the No-Build, Partial Build and Build alternatives, respectively.
3. The Newport Center Drive intersection is expected to operate at LOS F, E and C under the No-Build, Partial Build and Build alternatives, respectively. The Partial Build alternative would reduce the delay by 19 seconds/vehicle from the No-Build alternative and the Build alternative would further reduce it by more than 34 seconds/vehicle.
4. The I-95 SB Ramp intersection is expected to operate at LOS D, C and D under the No-Build, Partial Build and Build alternatives, respectively.
5. The I-95 NB Ramp intersection is expected to operate at LOS F, C and C under the No-Build, Partial Build and Build alternatives, respectively. The Partial Build and Build alternatives would reduce the overall intersection delays by more than 124 seconds per vehicle when compared with the No-Build alternative. The average vehicle delays between the Partial Build and Build alternatives are comparable.

6. The FAU Research Park Boulevard is expected to operate at LOS E, E and D under the No-Build, Partial Build and Build alternatives. The Partial Build would reduce the delay by more than 19 seconds per vehicle and the Build alternative would further reduce it by 5 seconds per vehicle.

VISSIM Simulation of the North and Center Build Alternative 3D-1.3

As discussed above, based on the VISSIM traffic simulation of the North and Center Build Alternatives, certain refinements were made to the Build concepts. Therefore, hereinafter, the 3D-1.3 Build Alternatives have been identified as “Base” and “Modified”. The modified alternative incorporates the elimination of the northbound and southbound through and left-turn movements from the intersection of SW 10th Street and Newport Drive and the conversion of the currently unsignalized intersection of SW 12th Avenue and Newport Drive into a roundabout.

Evaluation of North Build Alternative Base and Center Build Alternative Base

VISSIM micro-simulation analysis was conducted for the Base condition for North and Center Build Alternatives 3D-1.3. For the Center Base alignment, VISSIM micro-simulation analysis identified significant constraints in the WB direction between Military Trail and the I-95 ramps. The primary reason for the traffic congestion and backup was insufficient intersection throughput capacity at Newport Center Drive and constrained weaving operations accessing the WB express lane ingress. In the North Base alignment, the express lanes are relocated to the north side of SW 10th Street from the center location thereby providing better operating conditions than the Center Base alignment. **Tables 4A** and **4B** summarize the VISSIM network-wide summary. The green highlighting indicates the alternative with better operations. **Figures 7A** and **7B** depict the VISSIM network for the North Base and Center Base alternatives.

Findings

The VISSIM results indicate that in the Build Option 1.3 North Base, there is significant reduction in latent (i.e., unmet) demand and delay in the AM and PM conditions. Based on these traffic operations results the North Build Alternative Base performs better than the Center Build Alternative Base. Therefore, the Center Build Alternative was eliminated and North Build Alternative Base was advanced further in the PD&E process for additional concept refinements to improve the traffic operations at the Newport Center Drive intersection.

Evaluation of North Build Alternative Base and North Build Alternative Modified

To improve traffic operations and achieve acceptable traffic operations at all study area intersections, the North Build Alternative Base was further refined by eliminating the NB and SB through and left-turn movements at the Newport Center Drive and SW 10th Street intersection. The traffic from the eliminated movements was reassigned via the loop connector which passes under SW 10th Street along the railway line. **Tables 5A** and **5B** summarize the VISSIM network-wide summary. The green highlighting indicates the alternative with better operations. **Figures 8A** and **8B** depict the VISSIM network for the North Modified and Center Modified alternatives.

Findings

The VISSIM results indicate that in the North Build Alternative Modified, there is improvement in delay, speed and travel time in the AM condition. In the PM condition, there is significant reduction in latent (i.e., unmet) demand and delay along with improvement in delay, speed and travel time. Hence, the North Build Alternative Modified was operationally found to be the best alternative for further consideration in the PD&E process.

Table 1A - No-Build 2040 SW 10th Street Signalized Intersection Analysis Results - AM

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	AM Movement/Approach LOS (Delay)											Intersection AM LOS (Delay)	
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through		Right
SW 10th Street	South Military Trail	LOS (Delay)	Movement	F (95.2)	E (77.2)	C (32.4)	F (262.5)	F (152.7)	F (105.6)	F (85.6)	F (267.5)	F (452.8)	F (125.7)	E (66.5)	E (59.0)	F (151.2)
		Approach	E (76.8)			F (161.8)			F (317.7)			F (85.6)				
		Volume to Capacity ratio	Movement	0.84	1.07	0.13	1.42	1.12	0.38	0.7	1.41	1.81	1.03	0.75	0.49	
		Queue Length 95th (ft)	Movement	m229	m#1179	m20	#408	#1386	437	173	#912	#1205	#464	463	275	
	East Newport Center Drive	LOS (Delay)	Movement	F (150.1)	B (19.2)		F (448.2)	B (17.3)	A (8.2)	F (84.9)	F (84.9)	F (82.1)	F (84.8)	F (84.4)	F (82.3)	D (53.0)
		Approach	C (30.5)			E (77.5)			F (83.2)			F (83.2)				
		Volume to Capacity ratio	Movement	1.14	0.86		1.76	0.69	0.3	0.44	0.45	0.07	0.4	0.37	0.08	
		Queue Length 95th (ft)	Movement	m#311	m904		#998	840	176	87	89	71	72	72	59	
	I-95 Southbound On-ramp	LOS (Delay)	Movement		E (79.9)	A (0.5)	F (226.8)	A (0.2)								E (57.2)
		Approach	E (60.4)			D (50.4)										
		Volume to Capacity ratio	Movement		0.83	0.48	1.32	0.48								
		Queue Length 95th (ft)	Movement		694	0	#1603	0								
	I-95 Southbound Off-ramp	LOS (Delay)	Movement		A (9.8)			A (7.1)					E (56.3)		A (2.2)	E (57.2)
		Approach	A (9.8)			A (7.1)						B (18.6)				
		Volume to Capacity ratio	Movement		0.68			0.68					0.5		0.66	
		Queue Length 95th (ft)	Movement		706			m125					286		0	
	I-95 Northbound Ramps	LOS (Delay)	Movement		C (22.6)	A (7.4)	F (275.5)	D (41.1)		F (213.6)		F (261.0)				F (81.4)
		Approach	B (14.6)			E (72.1)			F (228.3)							
		Volume to Capacity ratio	Movement		0.65	0.91	1.38	0.81		1.3		1.38				
		Queue Length 95th (ft)	Movement		591	1290	#683	784		#879		#993				
FAU Research Park Boulevard	LOS (Delay)	Movement	F (126.4)	C (22.9)		C (23.2)	C (25.9)	B (17.3)	F (148.6)	E (58.0)	E (56.3)	E (75.3)	E (65.3)	F (118.8)	D (48.7)	
	Approach	D (38.4)			C (25.3)			F (95.8)			F (93.1)					
	Volume to Capacity ratio	Movement	1.09	0.55		0.68	0.61	0.06	1.09	0.27	0.1	0.84	0.64	0.99		
	Queue Length 95th (ft)	Movement	#428	410		124	476	20	#444	117	69	#372	292	#500		

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

*Combined SB ramps intersections delay notes:

The WBT at the I-95 SB on-ramp intersection and EBT at the I-95 SB off-ramp intersection are not used in the calculation of the combined weighted intersection delay.

Table 1B - No-Build 2040 SW 10th Street Signalized Intersection Analysis Results - PM

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	PM Movement/Approach LOS (Delay)											Intersection PM LOS (Delay)	
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through		Right
SW 10th Street	South Military Trail	LOS (Delay)	Movement	F (314.4)	E (61.2)	C (23.6)	E (78.7)	F (342.0)	E (55.4)	F (92.2)	E (68.3)	E (61.8)	F (85.3)	F (92.3)	F (96.5)	F (157.2)
		Approach	F (97.7)			F (263.3)			E (70.4)			F (92.5)				
		Volume to Capacity ratio	Movement	1.44	1.01	0.15	0.91	1.63	0.66	0.78	0.8	0.6	0.75	0.99	0.96	
		Queue Length 95th (ft)	Movement	#452	#958	m31	m291	m#2176	m338	#191	507	351	217	#758	#731	
	East Newport Center Drive	LOS (Delay)	Movement	F (83.0)	B (16.6)		F (208.8)	D (51.2)	B (14.8)	F (113.5)	F (115.7)	F (406.9)	E (65.1)	E (65.1)	F (262.3)	F (81.8)
		Approach	B (18.7)			E (57.5)			F (275.4)			F (215.5)				
		Volume to Capacity ratio	Movement	0.73	0.76		1.15	1.02	0.11	0.91	0.92	1.68	0.22	0.23	1.36	
		Queue Length 95th (ft)	Movement	m78	m229		#380	#1358	34	#408	#416	#824	122	123	#727	
	I-95 Southbound On-ramp	LOS (Delay)	Movement		F (88.5)	A (0.5)	F (101.0)	A (0.2)								D (45.3)
		Approach	E (66.3)			C (22.6)										
		Volume to Capacity ratio	Movement		1	0.48	1.03	0.48								
		Queue Length 95th (ft)	Movement		m625	m0	#843	0								
	I-95 Southbound Off-ramp	LOS (Delay)	Movement		B (12.6)			A (9.9)					D (54.0)		A (4.1)	D (45.3)
		Approach	B (12.6)			A (9.9)						B (14.4)				
		Volume to Capacity ratio	Movement		0.66			0.63					0.36		0.79	
		Queue Length 95th (ft)	Movement		m661			m151					205		0	
	I-95 Northbound Ramps	LOS (Delay)	Movement		D (52.5)	A (1.7)	E (75.3)	C (32.6)		F (441.1)		F (501.6)				F (148.4)
		Approach	C (31.5)			D (39.6)			F (460.0)							
		Volume to Capacity ratio	Movement		0.88	0.67	0.81	0.66		1.8		1.92				
		Queue Length 95th (ft)	Movement		818	380	#528	590		#1186		#1305				
FAU Research Park Boulevard	LOS (Delay)	Movement	D (40.1)	C (29.1)		F (147.0)	C (23.2)	B (17.6)	F (567.3)	E (57.8)	E (56.1)	E (77.2)	F (113.1)	F (136.8)	E (79.2)	
	Approach	C (30.4)			D (40.0)			F (321.3)			F (113.6)					
	Volume to Capacity ratio	Movement	0.87	0.72		1.11	0.49	0.09	2.08	0.25	0.09	0.85	0.99	1.05		
	Queue Length 95th (ft)	Movement	#272	601		#417	353	39	#655	107	63	#391	#531	#542		

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

*Combined SB ramps intersections delay notes:

The WBT at the I-95 SB on-ramp intersection and EBT at the I-95 SB off-ramp intersection are not used in the calculation of the combined weighted intersection delay.

Table 2A - Partial-Build 2040 - SW 10th Street Signalized Intersection Analysis Results - AM

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	AM Movement/Approach LOS (Delay)												Intersection AM LOS (Delay)
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
SW 10th Street	South Military Trail	LOS (Delay)	Movement	E (70.6)	F (105.7)	B (11.4)	E (56.0)	C (30.6)	A (2.0)	F (84.1)	F (190.5)	F (153.1)	F (174.5)	E (75.9)	E (62.9)	F (88.7)
			Approach	F (94.4)			C (27.4)			F (160.6)			F (108.3)			
		Volume to Capacity ratio	0.8	1.14	0.18	0.62	0.77	0.64	0.69	1.23	1.17	1.17	0.83	0.43		
		Queue Length 95th (ft)	m168	m637	m23	m247	460	7	#193	#695	#856	#500	438	213		
	East Newport Center Drive	LOS (Delay)	Movement	E (65.3)	B (10.1)		E (79.0)	B (18.3)	A (3.2)	F (84.9)	F (82.1)	F (81.9)	F (96.1)	F (82.3)	F (80.4)	C (23.9)
			Approach	B (16.1)			C (24.7)			F (83.1)			F (85.3)			
		Volume to Capacity ratio	0.69	0.83		0.82	0.8	0.25	0.52	0.12	0.08	0.65	0.31	0.07		
		Queue Length 95th (ft)	m210	m305		m312	m741	m2	85	37	72	122	87	30		
	I-95 Southbound Ramps	LOS (Delay)	Movement		E (65.5)	A (0.5)	F (91.3)	A (6.1)					D (45.8)		F (92.1)	D (45.5)
			Approach	D (48.7)			C (28.8)						E (77.1)			
		Volume to Capacity ratio		0.98	0.46	0.86	0.75					0.39		1.01		
		Queue Length 95th (ft)		#658	0	m411	m143					265		#816		
	I-95 Northbound Ramps	LOS (Delay)	Movement		A (0.6)	A (0.2)		B (10.2)	A (0.1)	F (96.0)		E (71.2)				C (23.2)
			Approach	A (0.4)			A (9.0)			F (87.5)						
		Volume to Capacity ratio		0.41	0.34		0.39	0.19	0.98		0.71					
		Queue Length 95th (ft)		m9	m11		235	m0	#496		300					
	FAU Research Park Boulevard	LOS (Delay)	Movement	E (63.5)	C (20.1)	B (20.0)	F (102.3)	D (46.7)	C (29.6)	E (79.7)	E (59.0)	E (56.9)	E (59.9)	E (77.4)	F (106.6)	D (50.5)
			Approach	C (27.1)			D (51.7)			E (68.5)			F (86.0)			
Volume to Capacity ratio		0.86	0.66	0.24	0.88	0.79	0.06	0.95	0.28	0.12	0.69	0.7	0.91			
Queue Length 95th (ft)		#275	278	57	#392	763	15	#481	147	68	306	325	#427			

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

Table 2B- Partial-Build 2040 - SW 10th Street Signalized Intersection Analysis Results - PM

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	PM Movement/Approach LOS (Delay)											Intersection PM LOS (Delay)	
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through		Right
SW 10th Street	South Military Trail	LOS (Delay)	Movement	F (157.0)	E (65.6)	B (14.7)	F (101.4)	F (87.9)	C (21.3)	F (161.6)	E (78.6)	D (49.8)	E (77.5)	E (63.7)	F (183.9)	F (85.3)
			Approach	E (75.1)			E (78.7)			F (86.9)			F (108.7)			
		Volume to Capacity ratio	Movement	1.14	1.03	0.19	1.06	1.1	0.59	1.08	0.83	0.57	0.79	0.81	1.23	
		Queue Length 95th (ft)	Movement	m#268	m798	m49	m#338	m#1052	m185	#290	407	407	#372	568	#1078	
	East Newport Center Drive	LOS (Delay)	Movement	F (82.3)	C (22.8)		F (130.8)	E (62.3)	A (6.5)	E (68.6)	E (58.1)	F (182.4)	E (65.8)	F (185.0)	F (92.8)	E (62.9)
			Approach	C (25.0)			E (63.2)			F (127.2)			F (124.0)			
		Volume to Capacity ratio	Movement	0.9	0.97		0.88	1.06	0.09	0.67	0.04	1.18	0.52	1.16	0.84	
		Queue Length 95th (ft)	Movement	m#69	m541		m#131	#1229	m12	329	40	#768	252	#598	#400	
	I-95 Southbound Ramps	LOS (Delay)	Movement		C (27.3)	A (0.4)	E (55.5)	B (19.1)					D (53.8)		F (88.5)	C (32.2)
			Approach	C (20.4)			C (29.7)						E (77.7)			
		Volume to Capacity ratio	Movement		0.94	0.54	0.81	0.57					0.35		0.96	
		Queue Length 95th (ft)	Movement		m580	m9	m296	622					206		#614	
	I-95 Northbound Ramps	LOS (Delay)	Movement		A (0.3)	A (0.2)		A (5.4)	A (0.2)	F (81.6)		F (92.3)				C (23.4)
			Approach	A (0.3)			A (4.6)			F (86.2)						
		Volume to Capacity ratio	Movement		0.44	0.39		0.33	0.21	0.91		0.95				
		Queue Length 95th (ft)	Movement		m6	m46		m115	m0	#464		#478				
FAU Research Park Boulevard	LOS (Delay)	Movement	F (89.9)	D (40.7)	B (19.6)	F (114.8)	D (42.5)	C (31.2)	F (117.7)	E (61.0)	E (59.1)	D (54.1)	F (118.7)	F (88.4)	E (59.9)	
		Approach	D (44.5)			D (54.1)			F (84.8)			F (88.7)				
	Volume to Capacity ratio	Movement	0.78	0.97	0.28	0.98	0.66	0.12	1.03	0.28	0.14	0.64	0.98	0.83		
	Queue Length 95th (ft)	Movement	m219	m#790	m80	#601	560	76	#563	152	80	341	#605	#450		

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

Table 3A - Build 3D-1.3 2040 - North Alignment - SW 10th Street Signalized Intersection Analysis Results - AM (Modified)

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	AM Movement/Approach LOS (Delay)											Intersection AM LOS (Delay)	
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through		Right
SW 10th Street	South Military Trail	LOS (Delay)	Movement	F (89.6)	E (56.4)	A (4.6)	C (29.2)	D (50.3)	B (11.1)	E (67.3)	F (100.8)	C (25.6)	F (109.1)	E (73.3)	A (0.3)	D (54.8)
			Approach	E (60.1)			D (37.5)			E (63.4)			E (69.5)			
		Volume to Capacity ratio	Movement	0.94	0.56	0.09	0.26	0.92	0.54	0.34	0.99	0.71	0.98	0.79	0.21	
		Queue Length 95th (ft)	Movement	m#290	m161	m6	110	765	120	151	#597	343	#455	397	0	
	East Newport Center Drive	LOS (Delay)	Movement	F (81.1)	A (9.9)		E (64.2)	A (6.0)	A (4.1)			E (59.1)			E (69.7)	B (18.5)
			Approach	B (18.6)			B (13.9)			E (59.1)			E (69.7)			
		Volume to Capacity ratio	Movement	0.78	0.62		0.65	0.62	0.37			0.18			0.3	
		Queue Length 95th (ft)	Movement	m258	m566		m308	m452	m93			79			79	
	I-95 Southbound Ramps	LOS (Delay)	Movement		E (65.2)	A (0.7)	E (73.7)	B (16.1)					D (36.8)		F (89.2)	D (46.5)
			Approach	D (47.0)			C (31.4)						E (75.0)			
		Volume to Capacity ratio	Movement		0.94	0.43	0.84	0.85					0.31		1.03	
		Queue Length 95th (ft)	Movement		#558	0	m479	m438					226		#980	
	I-95 Northbound Ramps	LOS (Delay)	Movement		A (0.9)	A (0.1)		A (8.6)	A (0.2)	F (106.5)		E (69.6)				C (27.0)
			Approach	A (0.6)			A (7.5)			F (94.4)						
		Volume to Capacity ratio	Movement		0.39	0.29		0.36	0.19	1.02		0.69				
		Queue Length 95th (ft)	Movement		m9	m31		110	m0	#547		298				
FAU Research Park Boulevard	LOS (Delay)	Movement	E (69.4)	C (26.6)	C (31.5)	F (86.2)	D (44.1)	C (30.2)	E (69.7)	E (55.8)	D (54.0)	E (60.4)	E (76.8)	F (103.2)	D (51.6)	
		Approach	C (34.0)			D (48.4)			E (62.1)			F (84.3)				
	Volume to Capacity ratio	Movement	0.8	0.54	0.22	0.72	0.71	0.07	0.91	0.25	0.12	0.7	0.7	0.9		
	Queue Length 95th (ft)	Movement	205	280	71	187	683	21	#444	138	64	285	324	390		

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

Table 3B - Build 3D-1.3 2040 - North Alignment - SW 10th Street Signalized Intersection Analysis Results - PM (Modified)

Arterial	Signal Controlled Intersections	Measure of Effectiveness (MOE)	Location	PM Movement/Approach LOS (Delay)											Intersection PM LOS (Delay)	
				Eastbound			Westbound			Northbound			Southbound			
				Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through		Right
SW 10th Street	South Military Trail	LOS (Delay)	Movement	F (98.4)	E (70.3)	E (63.8)	D (39.2)	D (45.5)	C (27.2)	F (94.7)	E (75.8)	C (24.3)	E (79.0)	E (74.4)	A (0.8)	D (54.0)
			Approach	E (79.7)			D (39.1)			E (64.2)			D (51.5)			
		Volume to Capacity ratio	0.84	0.58	0.26	0.48	0.82	0.62	0.85	0.84	0.41	0.81	0.91	0.42		
	Queue Length 95th (ft)	m#272	m236	m128	289	687	377	#264	453	266	#370	633	0			
	East Newport Center Drive	LOS (Delay)	Movement	D (36.1)	A (7.2)		E (63.2)	C (20.3)	A (8.5)			F (81.7)			E (70.8)	C (28.5)
			Approach	A (8.3)			C (21.9)			F (81.7)			E (70.8)			
		Volume to Capacity ratio	0.09	0.55		0.19	0.77	0.09			0.88			0.92		
	Queue Length 95th (ft)	m65	445		m87	m664	m10			369			450			
	I-95 Southbound Ramps	LOS (Delay)	Movement		E (57.1)	A (1.6)	E (72.9)	B (13.1)					D (45.2)		F (84.1)	D (42.9)
			Approach	D (39.6)			C (32.4)						E (73.9)			
		Volume to Capacity ratio		0.93	0.63	0.87	0.56					0.28		0.98		
	Queue Length 95th (ft)		646	633	355	265					188		#750			
	I-95 Northbound Ramps	LOS (Delay)	Movement		A (6.9)	A (0.2)		A (3.8)	A (0.2)	E (70.9)		F (112.0)				C (24.3)
			Approach	A (3.7)			A (3.2)			F (91.6)						
		Volume to Capacity ratio		0.4	0.42		0.32	0.21	0.73		1.02					
	Queue Length 95th (ft)		m139	m163		71	m0	330		#505						
	FAU Research Park Boulevard	LOS (Delay)	Movement	F (91.0)	C (32.2)	C (31.7)	F (86.1)	D (49.4)	D (36.0)	E (79.9)	D (53.3)	D (52.4)	D (51.6)	F (90.3)	E (72.9)	D (54.9)
			Approach	D (41.4)			E (55.5)			E (64.4)			E (72.4)			
Volume to Capacity ratio		0.79	0.75	0.23	0.82	0.71	0.11	0.92	0.22	0.15	0.66	0.89	0.72			
Queue Length 95th (ft)	m212	m514	m94	285	616	66	#481	133	71	316	501	359				

Synchro 9.2.914.6

LOS notes:

HCM 2000 level of service (LOS) and delay results from Synchro

Delay is in sec/veh units

 : LOS E reflecting at capacity operations

 : LOS F reflecting over capacity operations

Queue notes:

HCM methodology does not report queues, results are from Synchro outputs report

~: Volume exceeds capacity, queue is theoretically infinite

#: 95th percentile volume exceeds capacity

m: Upstream metering is in effect

Table 4A: 2040 AM Peak Period VISSIM Network-wide Summary

Alternative (2040 AM Peak Period)	Average		Total			Latent	
	Delay (sec/veh)	Speed (mph)	Travel Time (veh-hrs)	Delay (veh-hrs)	Vehicles Processed	Delay (veh-hrs)	Demand (veh)
Build Option 1.3 Center Base	395	25	3,879	1,627	44,437	2,461	3,327
Build Option 1.3 North Base	327	27	3,717	1,393	45,936	6	8

= Favorable alternative for the referenced measure of effectiveness

Table 4B: 2040 PM Peak Period VISSIM Network-wide Summary

Alternative (2040 PM Peak Period)	Average		Total			Latent	
	Delay (sec/veh)	Speed (mph)	Travel Time (veh-hrs)	Delay (veh-hrs)	Vehicles Processed	Delay (veh-hrs)	Demand (veh)
Build Option 1.3 Center Base	1,035	15	6,803	4,566	47,763	3,982	5,904
Build Option 1.3 North Base	428	24	4,388	1,935	48,814	1,018	1,392

= Favorable alternative for the referenced measure of effectiveness

Table 5A: 2040 AM Peak Period VISSIM Network-wide Summary

Alternative (2040 AM Peak Period)	Average		Total			Latent	
	Delay (sec/veh)	Speed (mph)	Travel Time (veh-hrs)	Delay (veh-hrs)	Vehicles Processed	Delay (veh-hrs)	Demand (veh)
Build Option 1.3 North Base	327	27	3,717	1,393	45,936	6	8
Build Option 1.3 North Modified	255	29	3,414	1,078	45,670	1	1

= Favorable alternative for the referenced measure of effectiveness

Table 5B: 2040 PM Peak Period VISSIM Network-wide Summary

Alternative (2040 PM Peak Period)	Average		Total			Latent	
	Delay (sec/veh)	Speed (mph)	Travel Time (veh-hrs)	Delay (veh-hrs)	Vehicles Processed	Delay (veh-hrs)	Demand (veh)
Build Option 1.3 North Base	428	24	4,388	1,935	48,814	1,018	1,392
Build Option 1.3 North Modified	293	27	3,830	1,326	48,911	154	243

= Favorable alternative for the referenced measure of effectiveness

Figure 7A: Build Option 3D-1.3 North Base



Figure 7B: Build Option 3D-1.3 Center Base

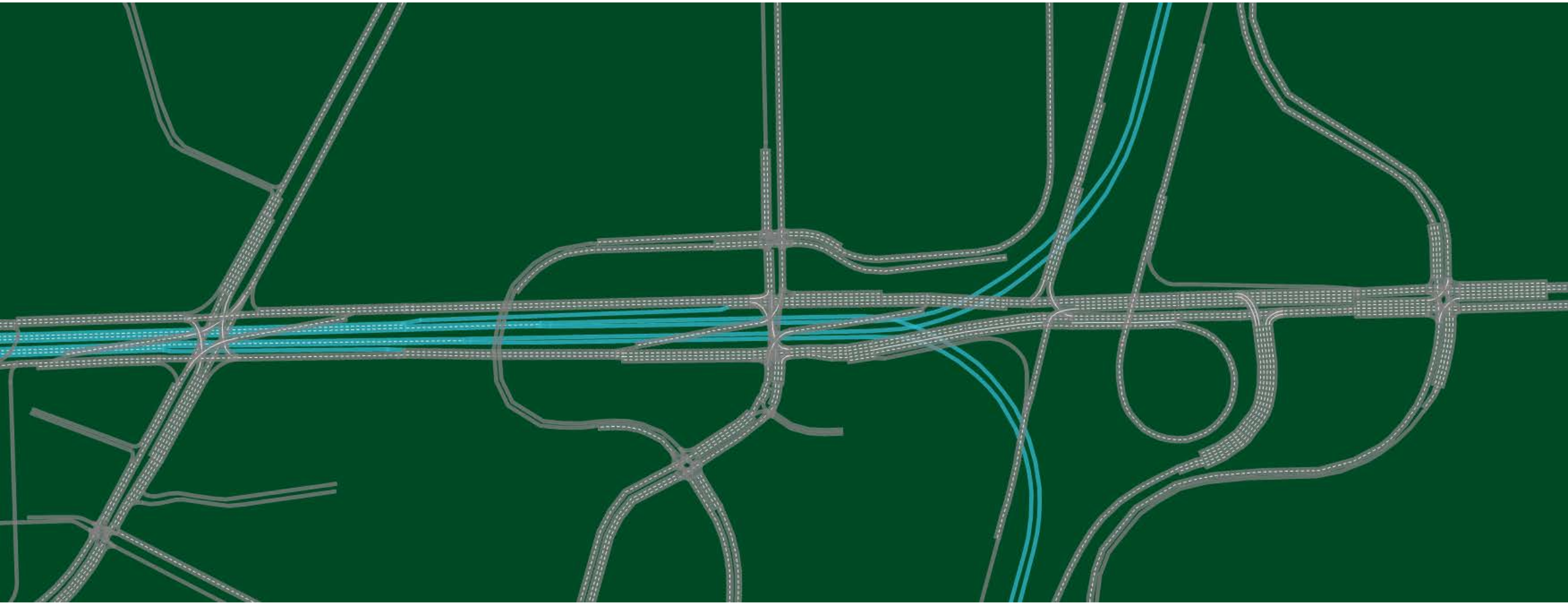


Figure 8A: Build Option 3D-1.3 North Modified



Figure 8B: Build Option 3D-1.3 Center Modified



Roundabout vs. Stop Controlled Intersection Analysis (SW 12th Avenue & Newport Center Drive)

The intersection of SW 12th Avenue and Newport Center Drive is currently a two-way stop controlled intersection with the NB and SB approaches as free movements and EB and WB movements as Stop controlled. In future years, it is projected that the truck activity at this intersection would increase causing potential safety concerns. Therefore, to mitigate any potential traffic operational safety concerns, a signal and a roundabout option were considered for this intersection. However, the signal warrant analysis was not satisfied for the intersection due to low traffic volume so the signal option was eliminated and a roundabout option was considered for traffic operational analysis comparison with the Stop controlled intersection.

Tables 6A and **6B** summarize the VISSIM analysis results for roundabout and Stop controlled intersection conditions. The VISSIM analysis indicates that the traffic operational analysis is very similar between roundabout and Stop controlled intersection. However, a roundabout has significant safety benefits over a Stop controlled intersection. According to FHWA – Technical Summary on Roundabouts, numerous studies have shown significant safety improvements at intersections converted from conventional forms to roundabouts. The physical shape of roundabouts eliminate crossing conflicts that are present at conventional intersections, thus reducing the total number of potential conflict points and the most severe of those conflict points. The most comprehensive and recent study showed overall reductions of 35 percent in total crashes and 76 percent in injury crashes. Severe, incapacitating injuries and fatalities are rare, with one study reporting 89-percent reduction in these types of crashes and another reporting 100-percent reduction in fatalities. Therefore, a roundabout is recommended at the intersection of SW 12th Avenue and Newport Center Drive.

North Build Alternative 3D-1.3 Modified – Evaluation of Eastbound Express Lane Egress Merge Conditions

Due to the proximity of the eastbound express lane egress to the Newport Center Drive intersection and the I-95 southbound ramp intersection, significant weaving and lane changes are induced in the eastbound direction. In order to provide the best operating conditions, the eastbound express lane egress was evaluated using VISSIM traffic simulation for the following merge conditions:

- Inside Egress Merge (see **Figure 9**)
- Outside Egress Merge (See **Figure 10**)
- Both Inside and Outside Egress Merge (See **Figure 11**)

Table 7 summarizes the results of the VISSIM analysis of the express lane egress merge conditions. The analysis shows that a combination of both inside and outside merge would provide the best operating conditions in the eastbound direction. **Figure 12** depicts the both inside and outside egress merge conditions with a roundabout at the intersection of Newport Center Drive and SW 12th Avenue.

Table 6A: 2040 AM Design Hour VISSIM Intersection Summary

Approach (AM Design Hour)	Movement	Roundabout			Stop Controlled		
		Queue Length Maximum (ft)	Demand Processed (Veh)	Delay (sec/veh)	Queue Length Maximum (ft)	Demand Processed (Veh)	Delay (sec/veh)
Northbound	U-Turn	0	0	0.0	0	0	0.0
	Left	55	24	10.3	36	25	1.8
	Through	55	30	8.3	0	31	0.1
	Right	55	6	7.3	9	6	0.7
Southbound	U-Turn	0	0	0.0	0	0	0.0
	Left	298	491	5.7	208	491	3.4
	Through	298	335	6.2	108	335	2.4
	Right	298	24	7.0	108	24	1.8
Eastbound	U-Turn	0	0	0.0	0	0	0.0
	Left	189	82	12.0	118	82	12.5
	Through	189	76	11.8	121	77	9.8
	Right	189	75	12.3	122	75	12.0
Westbound	U-Turn	0	0	0.0	0	0	0.0
	Left	39	4	2.9	73	4	11.5
	Through	39	66	2.2	89	66	14.0
	Right	39	32	1.6	89	32	5.6
Overall			1,246	6.9		1,247	5.2

Table 6B: 2040 PM Design Hour VISSIM Intersection Summary

Approach (AM Design Hour)	Movement	Roundabout			Stop Controlled		
		Queue Length Maximum (ft)	Demand Processed (Veh)	Delay (sec/veh)	Queue Length Maximum (ft)	Queue Length Maximum (ft)	Demand Processed (Veh)
Northbound	U-Turn	0	0	0.0	0	0	0.0
	Left	120	169	5.4	68	169	2.5
	Through	120	263	4.4	0	264	0.2
	Right	120	10	4.1	31	10	0.5
Southbound	U-Turn	0	0	0.0	0	0	0.0
	Left	127	94	12.6	84	93	3.8
	Through	127	54	13.7	0	54	1.9
	Right	127	28	15.6	0	28	2.8
Eastbound	U-Turn	0	0	0.0	0	0	0.0
	Left	80	176	3.5	126	175	13.7
	Through	80	9	4.4	128	9	7.3
	Right	80	9	4.0	130	9	10.5
Westbound	U-Turn	0	0	0.0	0	0	0.0
	Left	293	9	14.6	325	9	15.6
	Through	293	382	14.1	337	384	28.3
	Right	293	97	5.6	337	97	7.6
Overall			1,300	8.6		1,300	11.8

Figure 9: Build Option 3D-1.3 North Modified Inside Merge

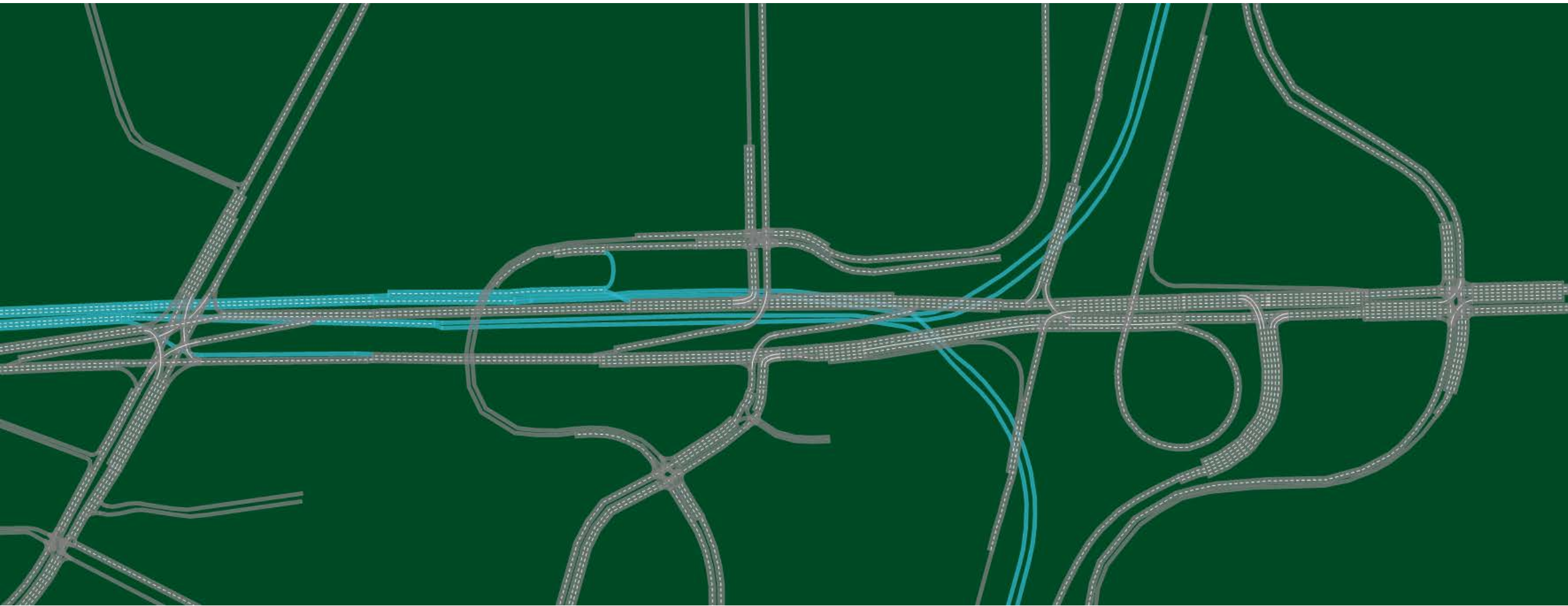


Figure 10: Build Option 3D-1.3 North Modified Outside Merge



Figure 11: Build Option 3D-1.3 North Modified Both Side Merge

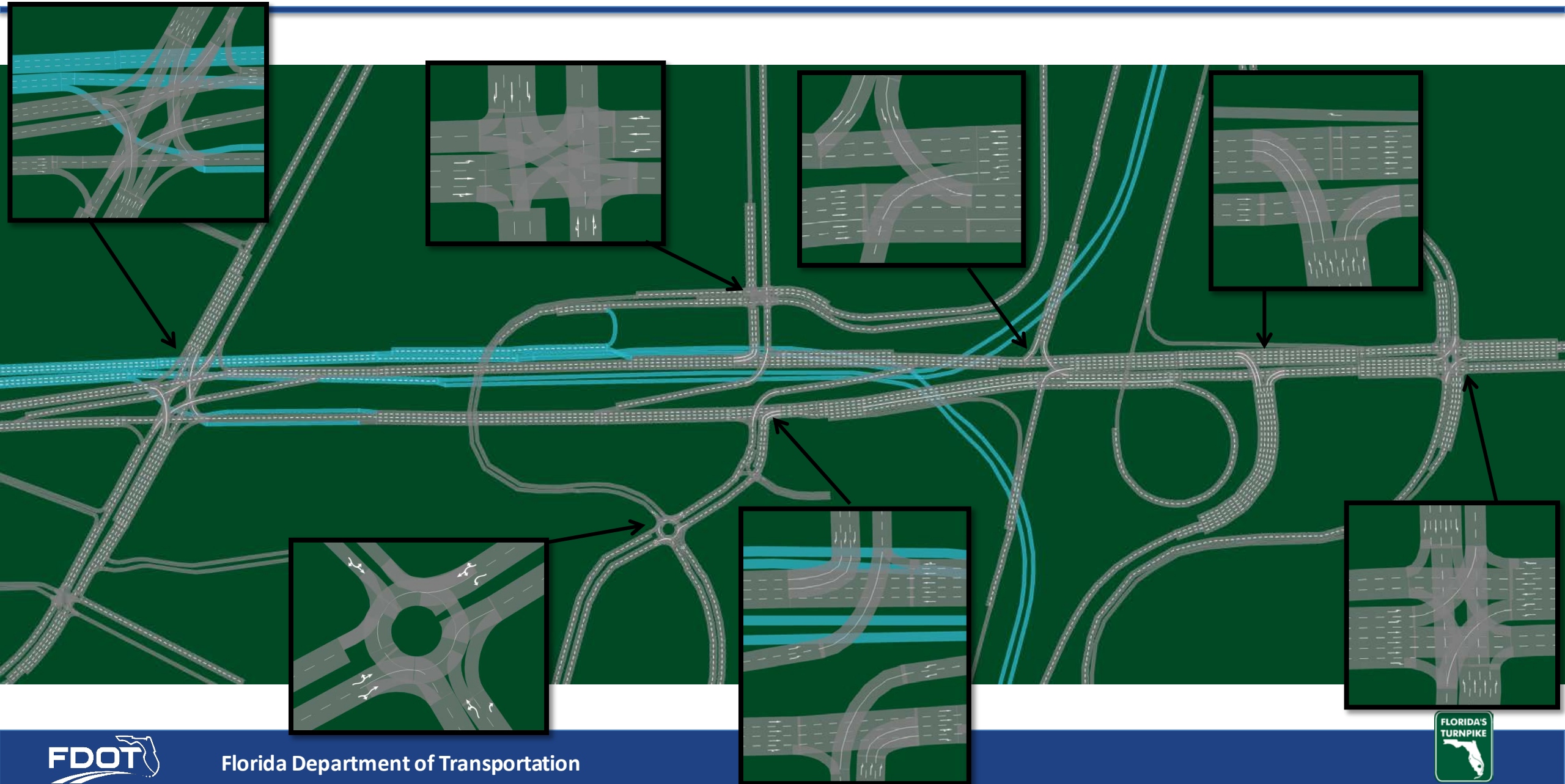


**Table 7: North Build Alternative 3D-1.3 Modified - Express Lane Alignment
Vehicle-Hours Comparison: Inside Egress Merge vs. Outside Egress Merge vs. Both Side Egress Merge**

Concepts	2040 (Delay in vehicle-hours)		
	AM	PM	Total
Inside Egress Merge	1,096	1,546	2,642
Outside Egress Merge	1,078	1,529	2,606
Both Sides Egress merge	1,083	1,406	2,489

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Figure 12: Build Option 3D-1.3 North Modified Both Side Merge With Roundabout



North Build Alternative 3D-1.3 Modified – Comparison of Weaving Volumes and Minimum Number of Lane Changes for the Eastbound Express Lane Egress Merge Conditions

In addition to the VISSIM analysis discussed above, the eastbound egress merge conditions were also evaluated for weaving volumes and the number of lane changes that would occur due to the different egress merge conditions in the eastbound direction. The origin and destination volumes for both the General Purpose (GP) lane and Express Lane (EL) were determined. Based on the location of the EL egress, the weaving volumes between the GP lane and the EL traffic were estimated. In addition, the number of lane changes that the vehicles from both the GP lane and the EL will have to make in order to reach their destinations was estimated. **Table 8** provides a comparison of the weaving volumes and minimum number of lane changes for the different express egress merge conditions. As shown in green highlighting, both sides egress merge would provide the best operating conditions. With both sides egress merge, the express lane traffic can reach their destinations without any weaving and lane changes, and the GP lane traffic would experience the least weaving and the least number of vehicles having to make lane changes to reach their destinations. **Figures 13** through **15** show the origin and destination traffic from both the GP and the Express lanes for the inside merge, outside merge, and both sides merge conditions, respectively.

Table 8: North Build Alternative 3D-1.3 Modified - Express Lane Alignment Weaving Volumes and Minimum Lane Changes Comparison: Inside Egress Merge vs. Outside Egress Merge vs. Both Side Egress Merge

Concepts	AM Peak Hour				PM Peak Hour			
	From GP Lane		From Express Lane		From GP Lane		From Express Lane	
	Weaving %	Minimum Lane Changes (vph)	Weaving %	Minimum Lane Changes (vph)	Weaving %	Minimum Lane Changes (vph)	Weaving %	Minimum Lane Changes (vph)
Inside Egress Merge	26%	356	54%	2,052	26%	300	61%	2,093
Outside Egress Merge	73%	1,269	38%	870	77%	1,167	41%	611
Both Sides Egress Merge	20%	267	0%	0	4%	48	0%	0

North Build Alternative 3D-1.3 Modified – User Benefit Calculation for the Eastbound Express Lane Egress Merge Conditions

A user benefit calculation was conducted for the different express egress merge conditions for design year 2040. Cumulative benefits for the three merge conditions in 2018 dollars were determined based on the benefits for each year from opening year 2020 to design year 2040. **Table 9** summarizes the user benefits in dollars between the three merge conditions.

Table 9: North Build Alternative 3D-1.3 Modified - Express Lane Alignment User Benefit in Dollars

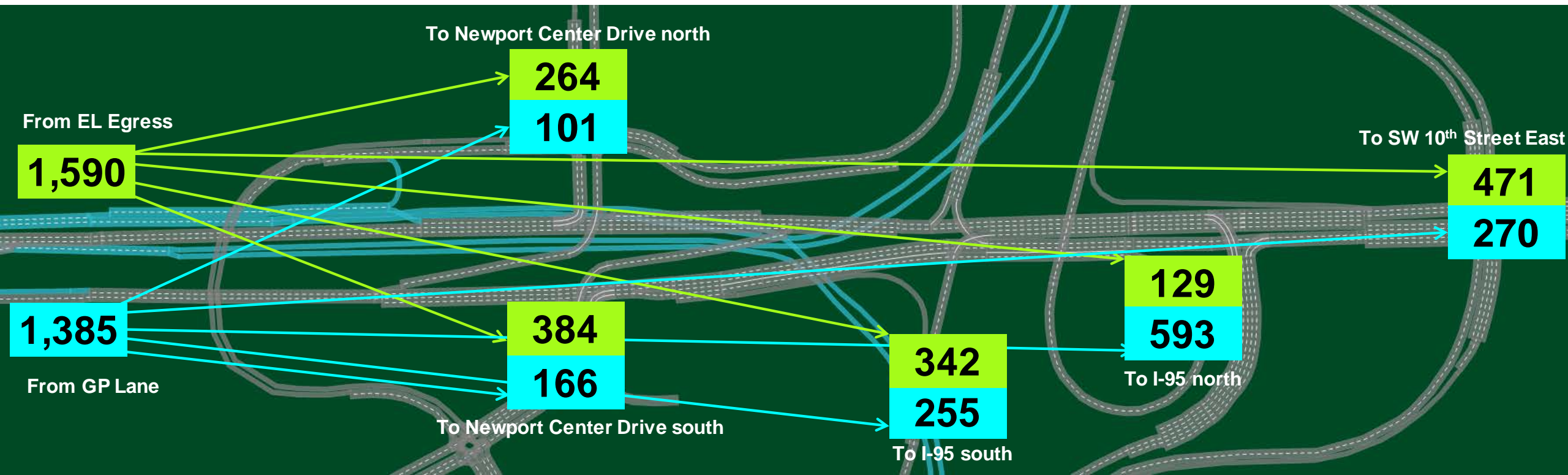
Present Day Benefit in Dollars (2018)	Outside Egress Merge Minus Inside Egress Merge	Both Sides Egress Merge Minus Inside Egress Merge	Both Sides Egress Merge Minus Outside Egress Merge
Cumulative Benefit Difference	\$1,513,100	\$6,441,200	\$4,927,500

The maximum benefit will be realized by both sides egress followed by outside egress and then inside egress. When compared with the inside egress, the outside egress provides approximately \$1.5 million in additional cumulative benefits. The both sides egress provides approximately \$6.4 million and \$4.9 million in additional benefits compared to the inside egress and outside egress, respectively. It can therefore be concluded from Table 9 that for as long as the cost of adding the inside merge to the outside merge remains within \$4.9 million, the benefits will outweigh the cost, otherwise it will be better to maintain the outside only option.

CONCLUSION

As documented in the *Traffic Analysis Technical Memorandum* dated May 4, 2018 and prepared by RS&H, Tier 1 and Tier 2 analyses resulted in the selection of the North Build Alternative 3D-1.3 and the Center Build Alternative 3D-1.3 as the two most suitable alternatives. VISSIM micro simulation resulted in the selection of the North Build Alternative 3D-1.3 as the operationally best Build Alternative. This alternative was further refined to eliminate the northbound and southbound through and left-turn movements from the SW 10th Street and Newport Center Drive intersection and convert the unsignalized intersection of SW 12th Avenue and Newport Center Drive into a roundabout. Additionally, three different express lane egress merge conditions were evaluated for weaving volumes, number of lane changes, and vehicle hours of delay. A combination of both the inside and outside express lane egress merge condition was found to provide the best operating conditions. The both sides egress merge condition also provided the maximum cumulative benefit when compared to the inside only and the outside only merge conditions.

Figure 13A: 2040 AM Build Condition – Inside Merge EB Weaving



EL

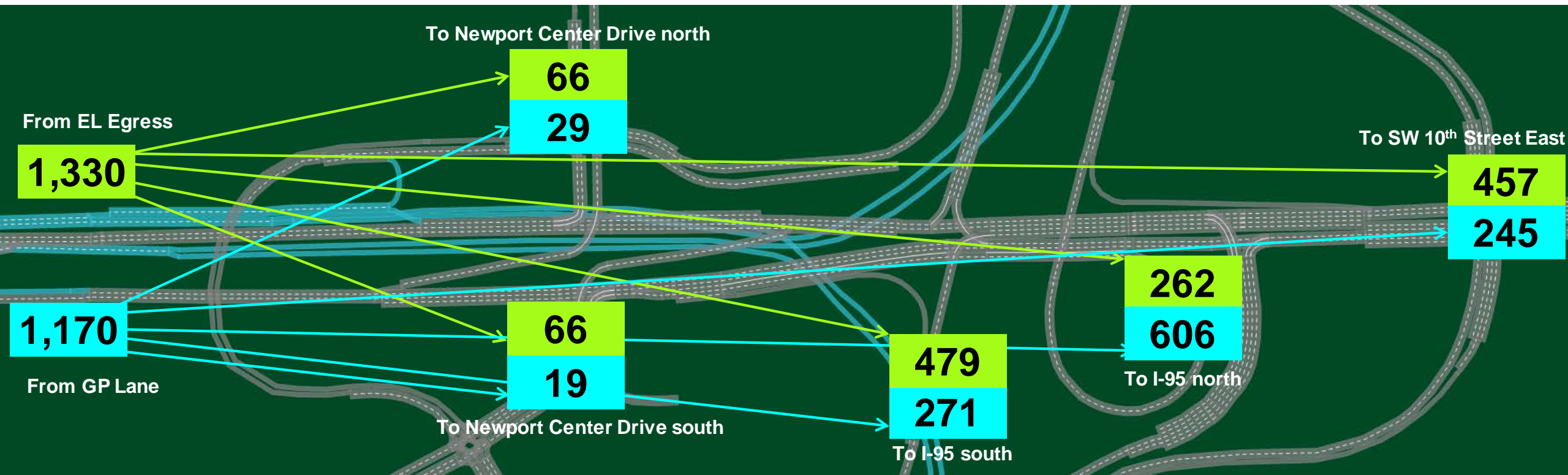
GP

Traffic volumes in vehicles per hour (vph)

54 percent express lane traffic weaving, and 2,052 vph minimum lane changes to reach destination

26 percent general-purpose lane traffic weaving, and 356 vph minimum lane changes to reach destination

Figure 13B: 2040 PM Build Condition – Inside Merge EB Weaving



EL

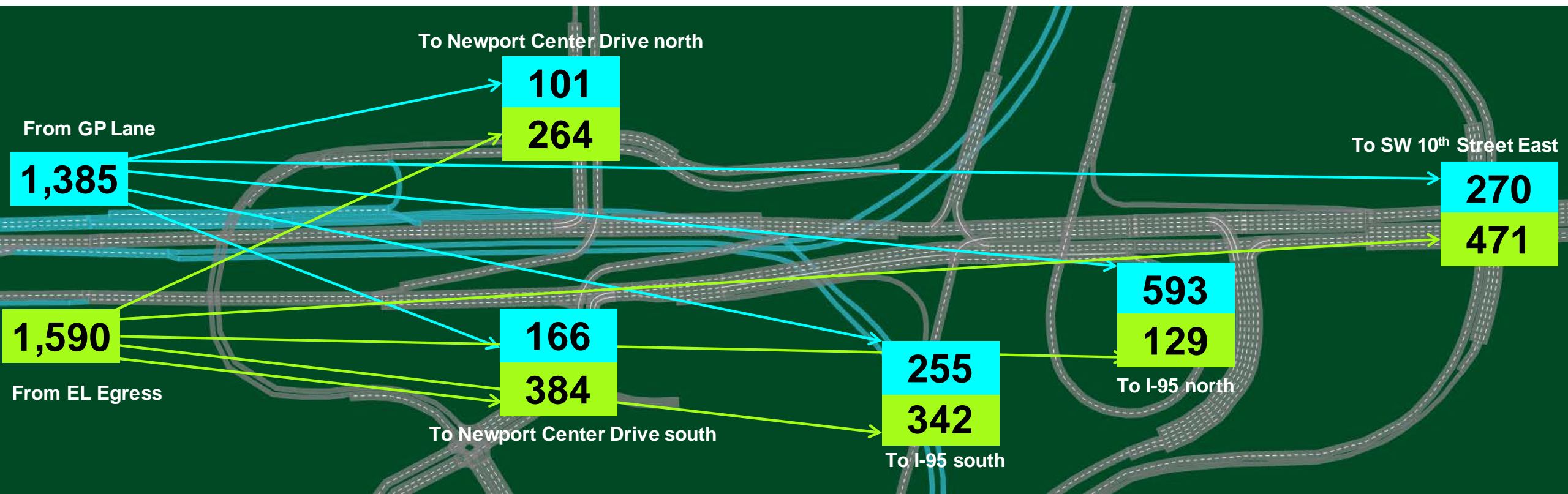
GP

Traffic volumes in vehicles per hour (vph)

61 percent express lane traffic weaving, and 2,093 vph minimum lane changes to reach destination

26 percent general-purpose lane traffic weaving, and 300 vph minimum lane changes to reach destination

Figure 14A: 2040 AM Build Condition – Outside Merge EB Weaving



38 percent express lane traffic weaving, and 870 vph minimum lane changes to reach destination

73 percent general-purpose lane traffic weaving, and 1,269 vph minimum lane changes to reach destination

EL

GP

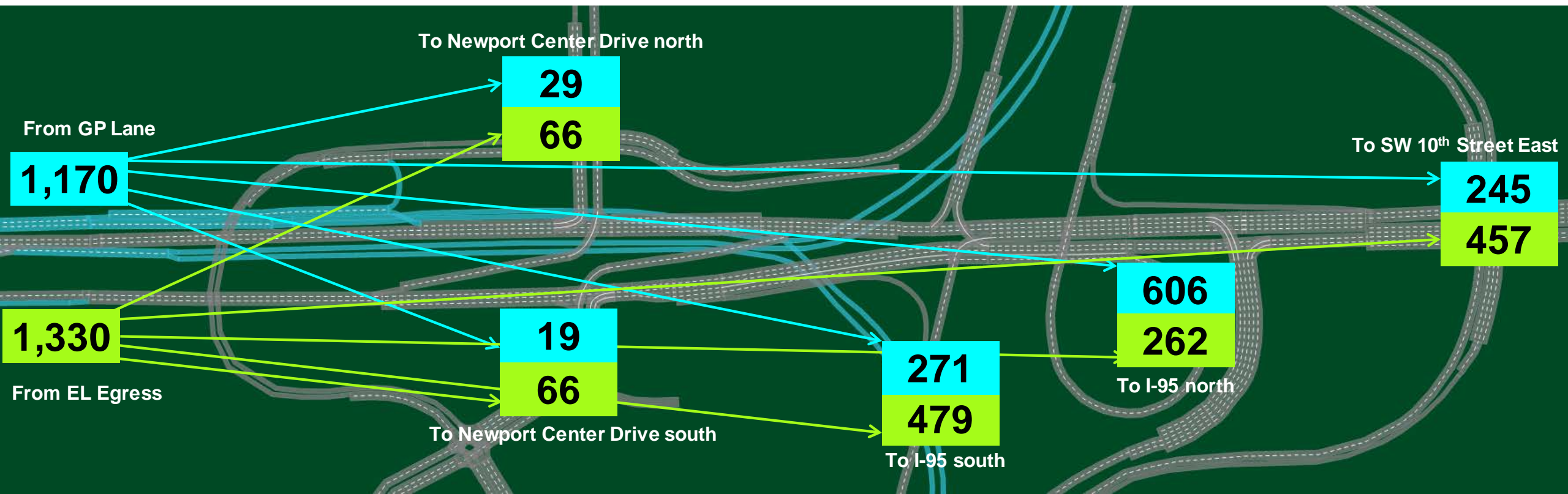
Traffic volumes in vehicles per hour (vph)



Florida Department of Transportation



Figure 14B: 2040 PM Build Condition – Outside Merge EB Weaving



41 percent express lane traffic weaving, and 611 vph minimum lane changes to reach destination

77 percent general-purpose lane traffic weaving, and 1,167 vph minimum lane changes to reach destination

EL

GP

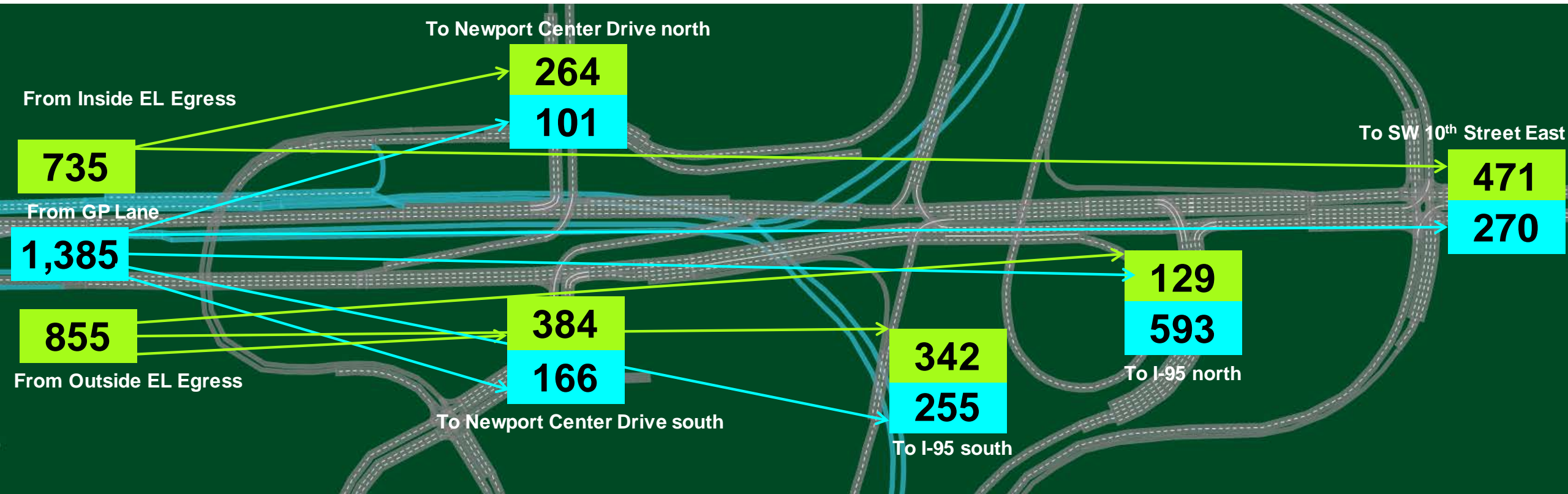
Traffic volumes in vehicles per hour (vph)



Florida Department of Transportation



Figure 15A: 2040 AM Build Condition – Both Side Merge EB Weaving



EL

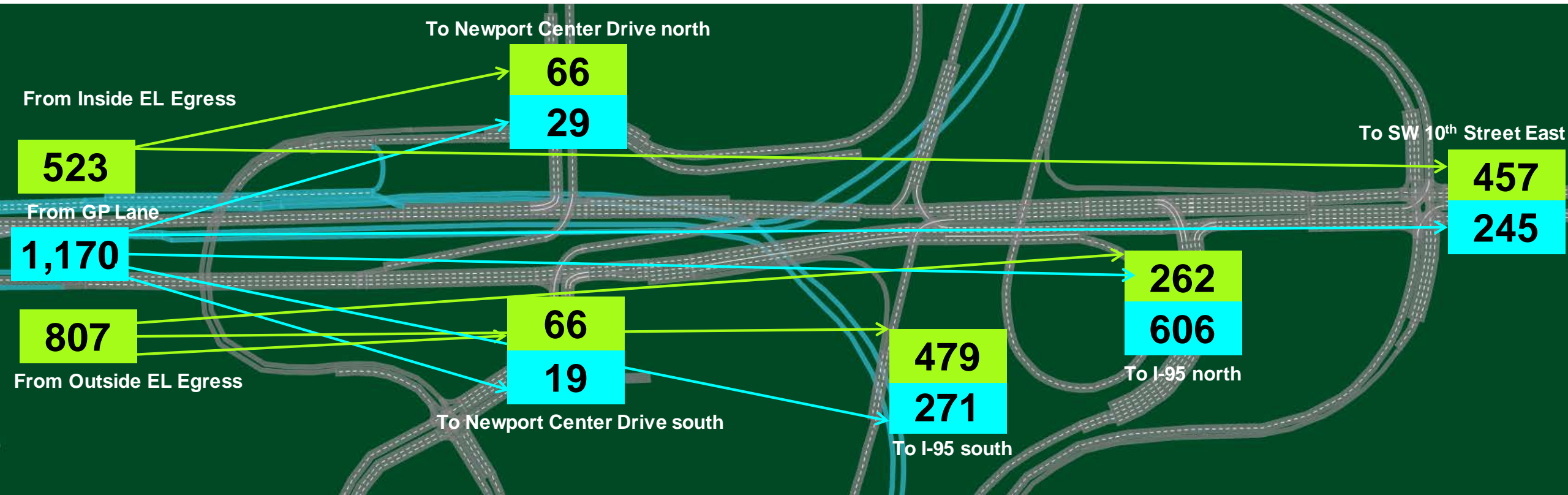
GP

Traffic volumes in vehicles per hour (vph)

0 percent express lane traffic weaving, and 0 vph minimum lane changes to reach destination

20 percent general-purpose lane traffic weaving, and 267 vph minimum lane changes to reach destination

Figure 15B: 2040 PM Build Condition – Both Side Merge EB Weaving



EL

GP

Traffic volumes in vehicles per hour (vph)

0 percent express lane traffic weaving, and 0 vph minimum lane changes to reach destination

4 percent general-purpose lane traffic weaving, and 48 vph minimum lane changes to reach destination