"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

FDOT FRP Rebar Implementation - Current status and updates **Steven Nolan, PE.**

Florida Department of Transportation

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Speaker Bio: Steven Nolan, P.E.



Professional Engineer in Florida since 2003, current technical lead coordinator for Florida DOT for implementation of Fiber-Reinforced Polymer reinforcing and prestressing, stainless-steel prestressing, and UHPC for structural applications. 10-years' experience with development of design guidance for FRP, 30-years' experience with concrete design and construction including 25-years with bridge design specification and standards development. Current member of TRB committee AKB10-Innovative Highway Structures, ACI 440C & CSAO, ASCE-Structural Engineering Institute, Bridge Engineering Institute, and *fib* (International Federation for Structural Concrete).



Background:

- FDOT introduced guidance for the implementation of Fiber-Reinforced Polymer-Reinforced and Prestressed Concrete (FRP-RC & FRP-PC) at the 2014 Design Training Expo. We highlighted the planned release of Standard Specifications, Structures Manual, and Materials Manual updates, and the early design work for the seminal demonstration project - <u>Halls River Bridge</u>.
- Reflecting on 10 years of implementation and the evolution of design guidance, standard specifications and plans, many projects have now been successful completed and continue to be monitored with the goal of improving the state-of-the-practice and cost efficiency.



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• 2014 FDOT (Invitation to Innovation)

WELCOME TO THE STRUCTURES DESIGN OFFICE

Office Manager: Will Potter, P.E. - State Structures Design Engineer

The Structures Design Office provides design guidance and technical assistance for structural, geotechnical, mechanical and electrical issues related to structural design and construction. The Structures Design Office is 1 of 3 divisions under the **Office of Design**, along with **Roadway Design**, and the **CADD Office**.









Structures Design Office



Fiber Reinforced Polymer Reinforcing

Structures Design - Transportation Innovation Fiber Reinforced Polymer (FRP) Reinforcing Bars and Strands

Overview Usage Restrictions / Parameters Design Criteria Specifications Standards Producer Quality Control Program Projects Technology.Transfer (T²) FDOT Research Contact



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CFRP Prestressing Strand & more... at FDOT





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Design: Practices and Standards for FRP-RC/PC





FRP Reinforced Concrete Structures: 10-year update, Lessons Learned, and Emerging Best Practices

Steven Nolan, P.E. State Structures Design Office, FDOT (Tallahassee)

FRP Reinforced Concrete Design

Presented in 2017 by: Updates in 2024 by:

FDOT

y:Rick Vallier, P.E.oy:Steven Nolan, P.E.

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2017 vs. 2024 FRP Reinforced Concrete Outline



- A. FRP Reinforcing Bars
- B. Research
- C. Structures Manual
- **D. Design Standards**
- E. Developmental Design
 - **Standards**
- E. Standard Specifications

F. Challenges



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A. Reinforcing Bars

Spec. 932-34.2 Bar Sizes and Loads

		Table 932-8						
Par Siza	Nominal	Nominal Nominal	Measured Cross-Sectional Area (in ²)		Minimum Guaranteed Tensile Load (kips)			
Designation	nDiameter (in)	Sectional Area (in ²)	Minimum	Maximum	BFRP & GFRP Bars (Type 0)	BFRP & GFRP Bars (Type III)	CFRP (Type II) Single & 7-Wire Strands	CFRP <mark>(Type I)</mark> Bars
2.1-CFRP	0.21	0.028	0.026	0.042	-	-	7.1	-
2	0.250	0.049	0.046	0.085	6.1	7.4	-	10.3
2.8-CFRP	0.280	0.051	0.048	0.085	-	-	13.1	-
3	0.375	0.11	0.104	0.161	13.2	16.0	-	20.9
3.8-CFRP	0.380	0.09	0.087	0.134	-	-	23.7	-
4	0.500	0.20	0.185	0.263	21.6	27.9	-	33.3
5	0.625	0.31	0.288	0.388	29.1	40.8	-	49.1
6	0.750	0.44	0.415	0.539	40.9	57.3	-	70.7
6.3-CFRP	0.630	0.19	0.184	0.242	-	-	49.8	-
7	0.875	0.60	0.565	0.713	54.1	75.8	-	-
7.7-CFRP	0.770	0.29	0.274	0.355	-	-	74.8	-
8	1.000	0.79	0.738	0.913	66.8	94.9	-	-
9	1.128	1.00	0.934	1.159	82.0	115.0	-	-
10	1.270	1.27	1.154	1.473	98.2	138.7	-	-
11	1.410	1.56	1.500	1.700	105.8	160.0	-	-



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A. Reinforcing Bars

Characteristics of FRP Reinforcement:

- Polymer resin matrix relatively weak:
 - Bond force is transferred through resin to fibers.
 - Shear resistance is considered relatively weak (~60%).
- Low compressive strength of FRP:
 - Design contribution of FRP reinforcement to resist compression is not recommended ignored.
- Modulus of elasticity is low:
 - Due to lower stiffness (6.5 *msi* & 8.7 *msi*), serviceability often controls the design.
- Creep-rupture threshold is low $(\frac{25\%}{30\%} f_u)$:
 - Sustained high tension can cause fibers to fail after a period of time
 - GFRP is considered more susceptible than CFRP (70% f_u).



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Tensile Strain (%)

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A. Reinforcing Bars

FRP Bar <u>Mechanical Characteristics</u> Influenced By:

Pre-Construction

- Manufacturing Process (FDOT MM Chapter 12.1)
- o Rate of Curing

Quality and Quantity of Constituents

Construction and Post-Construction

- Moisture (current limitation on BFRP in submerged marine environments. See **FRPG** 2.1)
- Oltraviolet Exposure (Spec. 416 limits on exposure)
- Elevated Temperature (Fire < T_g)
- Alkaline, Acidic, Saline Solutions ($C_E = 0.70$)



A. Reinforcing Bars

Characteristics of FRP Reinforcement:

- Endurance time in fire or elevated temperature less than for steel for anchorage zones (ACI 440-H is working on criteria):
 - Reinforcement type, aggregate type, and concrete cover will influence fire performance
 - Tensile, compressive, and shear properties of the resin material diminish as temperature approaches the glass transition temp. (*Tg*)

Property	Test Method	Requirement
Glass Transition	ASTM E1640 (DMA) or	≥230°F
Temperature (Tg)	ASTM E1356 (DSC)	≥212°F



https://www.fdot.gov/programmanagement/specs.shtm

A. Reinforcing Bars

Characteristics of FRP Reinforcement:

- <u>Life cycle costs likely</u> lower where steel corrosion is a concern (see HRB).
- <u>SCMs</u> (HRPs) for corrosion protection are may not be needed:
 - Silica Fume Ultrafine Fly Ash
 - Metakaolin
 <u>
 - Calcium Nitrite</u>
- <u>Transportation costs</u> are lower and handling easier for FRP due to light weight (~25%).
- <u>Concrete cover</u> reduction is allowed (see FRPG Table 2.2).



A. Reinforcing Bars

Bent Bars Characteristics:

- FRP is pultruded from thermoset resin (viable thermoplastic resins are emerging)
- FRP is fabricated with bends (thermoplastic and olefin resin will allow controlled thermal bending – <u>NCHRP IDEA-207</u> validated this):
 - Sharp bends can be manufactured, but avoided due to potential low stress failure.
 - Bend Radius / Bar Diameter ≥ 3.
 - Tail Length = 12 x Bar Diameter.
 - Field bending not permitted.

o Developmental Design Standard

Plan Index 415-010 D21310 Bar Bending Details



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A. Reinforcing Bars

From Developmental Design Standard Plans Index D21310 415-010:

(renumbering to match steel bar bending Index)





A. Reinforcing Bars

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Cost Comparison (Installed Price)

Bar	Nominal Diameter	Average Unit Costs HRB (3-bids) / **		FDOT Structures Manual for BDR Cost Estimating / **		
Size		GFRP Bars 2016	GFRP Bars 2023	Grade 60 Steel	Stainless-Steel	
#4	0.500"	\$1.18 / LF	\$1.90 / LF	\$ 0.60 1.11 / LF	\$ 2.72	
#5	0.625"	\$1.37 / LF	\$2.29/ LF	\$ 0.94 1.74 / LF	\$ 4.19	
#6	0.750"	\$1.55 / LF	\$2.71/ LF	\$ 1.35 2.51 / LF	\$ 5.98 11.54 / LF	
#8	1.000"	\$2.54 / LF	\$4.04/ LF	\$ 2.40 4.45 / LF	\$ 10.74 20.73 / LF	

Note: There is not 1:1 substitution of FRP for steel bars. Black steel bar based on \$1.67**\$0.90 / lb for all bar sizes. Stainless steel bar based on \$7.72**\$4.00 / lb for all bar sizes.

** 2023 FDOT Bid Avg.

A. Reinforcing Bars

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Flexural Strength Design Philosophy

Steel Reinforced Concrete Design

- Tension-Controlled Behavior
- Yielding of Steel Prior to Concrete Crushing Provides Ductility and Warning of Distress through extensive cracking and deformation

FRP Reinforced Concrete Design

- Tension-Controlled Behavior
 - FRP Rupture *(phi = 0.55)*
- Compression-Controlled Behavior
 - Concrete Crushing prior to FRP Rupture (phi = 0.65 0.75)
- $\circ~$ Margin of Safety is Higher than for Steel Reinforced Design



A. Reinforcing Bars

Design Assumptions

- Plane sections remain plane
- Flexural strength using equivalent rectangular concrete stress distribution
- \circ Compressive strain in concrete assumed to be 0.003
- Tensile strength of concrete is ignored
- Perfect bond exists between concrete and FRP reinforcement
- Tensile behavior of FRP reinforcement is linear elastic until failure
- Compressive strength of FRP reinforcement is ignored



B. FDOT Research

Research and field implementation of FRP materials is ongoing and design recommendations continue to evolve and improved.

	Complete d	Title	Researcher	Institution	Research No.
	Nov. 2018	Performance Evaluation of GFRP Reinforcing Bars Embedded in Concrete Under Aggressive Environments	R. Kampmann	FAMU-FSU	BDV30 977-18
	April 2019	Degradation Mechanisms and Service Life Estimation of FRP Concrete Reinforcements	A. El Safty	UNF	BDV34 977-05
	6/30/2019	Performance Evaluation of Basalt Fiber Reinforced Polymer (BFRP) Reinforcing Bars Embedded in Concrete	R. Kampmann	FAMU-FSU	BVD30 986-01
	April 2022	Epoxy Dowel Pile Splice Evaluation with FRP Bars	A. Mehrabi	FIU	BDV29 977-52
	Dec. 2020	<u>"Stainless Steel Strands and Lightweight Concrete for</u> <u>Pretensioned Concrete Girders</u> (w/ GFRP shear stirrups)	M. Roddenberry	FAMU-FSU	BDV30 977-27 (Report A)
ន	07/30/2022	Improving Testing Protocol and Material Specifications for Basalt Fiber Reinforced Polymer Bars	R. Kampmann	FAMU-FSU	BE694

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B. FDOT Research - Completed and In-progress

Research and field implementation of FRP materials is ongoing and expanding...

	Completio n Date	Title	Researcher	Institution	Research No.
	08/29/2022	Development of GFRP Reinforced Single Slope Bridge Rail	G. Consolazio	UF	BDV31 977-110
	12/31/2023	Evaluation of Glass Fiber Reinforced Polymers (GFRP) Spirals in Corrosion Resistant Concrete Piles	S. Jung	FAMU-FSU	BDV30 977-27
	Feb. 2025	FSBs With Stainless Steel Strands and GFRP Shear Reinforcement	M. Roddenberry	FAMU-FSU	BED30 977-09
	April 2025	Waterline Pile Cap Footings for Bridges using Large Diameter FRP Reinforcing – Material Characterization and Design	A. Nanni	UM	BEE76 977-01
ន	May 2025	HRB Extraction and Physio-Mechanical Testing of FRP Reinforcing Bars from 5-year-old Seawater Concrete Test Blocks on Halls River Bridge Bulkhead	F. De Caso	UM	BEE76 977-02

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C. FDOT Structures Manual



FDOT STRUCTURES MANUAL

- Volume 1 Structures Design Guidelines
- Volume 2 Structures Detailing Manual

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- Volume 3 FDOT Modifications to LRFDLTS-1
- Volume 4 Floer Reinforced Polymer Guidelines

FDOT Design Criteria for FRP:

Vol. 1 – SDG

- Bearing Piles 3.5
- Fender Systems 3.14
- o Structural FRC-3.17
- BDR Cost Estimating 9.2
 - o Bearing Piles
 - o Sheet Pile
- Vol. 2 SDM
- Fender Systems 24

Vol. 4 – FRPG

- Reinforcing Bars 2
- \circ Strands 3
- \circ Strengthening 4
- Pultruded Shapes 5
- VIP Shapes 6
- Thermoplastic Shapes 7

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C. FDOT Structures Manual

FDOT Design Criteria for using FRP Composites:

The Structures Manual implements *basic design guidelines* for FRP composites in specific applications.

As is the case with all structural materials, the engineer must practice the *appropriate standard* of care when designing components using FRP composites.





C. FDOT Structures Manual

Volume 4 - Fiber Reinforced Polymer Guidelines (FRPG)

Unless otherwise stated within the *FRPG*, the use of FRP composites requires approval of the State Structures Design Office.

Obtain concept approval before proceeding with any design effort.

After concept is approved, submit the design to the State Structures Design Office for review.

FDM 121.3.2: *"Any component designed using Fiber Reinforced Polymer (FRP) composite materials except components in the Standard Plans"* is designated as **Category 2 Structure.**



C. FDOT Structures Manual

Volume 4 - Fiber Reinforced Polymer Guidelines (FRPG) – Section 2: Permitted use when approved by the SSDE:

- Approach Slabs
- Bridge Decks & Bridge Overlays
- Cast-in-Place Flat Slab
 Superstructure
- Pile Bent Caps not in direct
 contact with water
- Pile Jackets

 Pier Columns and Caps not in direct contact with water

- Retaining Walls, Noise Walls,
 Perimeter Walls
- ⊖ Traffic Railings
- Pedestrian/Bicycle Railings
- Bulkheads and Bulkhead
 Copings
- MSE Wall Panels and Copings
- Drainage Structures
- Dowel Bars for Exp. Joints

Note: Other locations will be considered on a case-by-case basis.

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C. FDOT Structures Manual

FDOT Structures Manual – Vol. 4 FRPG 2.3

Concrete Cover Requirements in Extremely Aggressive Environments

Component	FRP Cover Requirements	Steel Cover Requirements
External Surface Cast Against Earth	3 in.	4.5 in.
Box Culverts	2.5 2 in.	3 in.
C.I.P. Cantilever Retaining Walls	2.5 2 in.	3 in.
MSE Walls	2 1.5 in.	3 in.
Bulkheads and Sheet Pile Caps	3 2 in.	4 in.

See FDOT Structures Manual for cover requirements for other components.



C. FDOT Structures Manual

GFRP/CFRP Reinforcing Bars – Section 2 – Design Criteria

Design **concrete members** with FRP reinforcement according to:

- AASHTO LRFD Bridge Design Guide Specifications for GFRP-Reinforced Concrete ACI 440.1 Guide for the Design and Construction of Structural Concrete Reinforced with FRP Bars.
- AASHTO Guide Specification for the Design of Concrete Bridge Beams Prestressed with CFRP Systems. ACI 440.4 Prestressing Concrete Structures with FRP Tendons

Design Bridge Decks according to:

 AASHTO LRFD Bridge Design Guide Specifications for GFRP-Reinforced Concrete Bridge Decks and Traffic Railings (2nd Edition)

Use FRP Mechanical Properties per FDOT Section 932-4.



Comparison of Rebar Qty. – Bridge Deck Example A



Comparison of Rebar Qty. – Bridge Deck Example A



Convention CS-RC (ASTM A615):

- Thickness = 8"
- Concrete Cover = 2"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical (AASHTO-BDS): Bottom layers
 = #5's @ 13.8" (0.27 in²/ft); Top layers
 = #4's @ 13.3" (0.18 in²/ft)
- Empirical (*FDOT*): #5's @ 12"**
- Total Rebar (AASHTO): $A_s = 0.90 \text{ in}^2/\text{ft}^2$; (FDOT): $A_s = 1.24 \text{ in}^2/\text{ft}^2$



<u>GFRP-RC</u> (ASTM D8505-23):

- Thickness = 7" $\rightarrow \Delta_{vol.}$ = -12%; Δ_{weight} = -15%
- Concrete Cover = 1"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical (AASHTO-GSG): Bottom transverse layer = #5's @ 6.5" (0.83%) Other 3-layers = #5's @ 12" (0.35%)
- Total Rebar: $A_f = 1.50 \text{ in}^2/\text{ft}^2 \text{ deck}$ $\rightarrow \Delta_{\text{volume}}$ (AASHTO) = +67%; $\Delta_{\text{weight}} = -58\%$ $\rightarrow \Delta_{\text{volume}}$ (FDOT) = +21%; $\Delta_{\text{weight}} = -70\%$

Interior Section

[•] Minimum bar size and spacing governs

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______ ____ Q_ BEAM 1R

Convention CS-RC (ASTM A615):

- Thickness = 8"
- Concrete Cover = 2"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical: (AASHTO): Bottom layers = #5's @ 13.8" (0.27 in²/ft); Top transverse = #4's @ 13.3" (0.18 in²/ft); + Top long. = #5's @ 6" (0.62 in²/ft)
- Empirical: (FDOT): 3-layers @ #5's @ 12" ** (0.31 in²/ft);
- Total Rebar (*AASHTO*): A_s = 1.34 in²/ ft² (*FDOT*): A_s = 1.55 in²/ ft²

Link-Slab Section



<u>GFRP-RC</u> (ASTM D8505-23):

- Thickness = 7" $\rightarrow D_{vol.}$ = -12%; D_{weight} = -15%
- Concrete Cover = 1"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical (FDOT/AASHTO-GSG): **Bottom transverse = #5's @ 6.5" (0.83% = 0.57 in²); Other 2-layers = #5's @ 12" (0.35% = 0.31 in²); + Top long. = #5's @ 6" (0.62 in²/ft)
- Total Rebar: $A_f = 1.81 \text{ in}^2/\text{ft}^2$ $\rightarrow \Delta_{vol.}$ (AASHTO) = +35%; $\Delta_{weight} = -66 \%$ $\rightarrow \Delta_{vol.}$ (FDOT) = +17%; $\Delta_{weight} = -71 \%$

G BEAM 3R

2" CLR. (BOT.)

— @ BFAM 4R

Convention CS-RC (ASTM A615):

- Thickness = 8"
- Concrete Cover = 2"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical (AASHTO-BDS): 2~Bot. layers = #5's @ 13.8" (0.27 in²/ft); Top long. = #4's @ 13.3" (0.18 in²/ft); Top transv. = #4's + #5's @ 4" (0.82 in²/ft).
- Empirical (FDOT): 3-layers #5's @ 12" ** (0.31 in²/ft); Top Tranv. = #5's @ 4"(0.93 in²/ft).
- Total Rebar (AASHTO): A_s = 1.54 in²/ft²; (FDOT): A_s = 1.86 in²/ft².

Overhangs (midspan)

<u>**GFRP-RC**</u> (ASTM D8505-23):

- Thickness = 7" → Δ_{vol.} = 88%; Δ_{weight} = 85%
- Concrete Cover = 1"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical (AASHTO-GSG): Bot. ** transv. = #5's @ 6.5" (≥0.83% = 0.57 in²); 2~long. layers = #5's @ 12" (≥0.35% = 0.31 in²); Top transv. = #5's @ 4" (0.93 in²/ft).
- Total Rebar: $A_f = 2.12 \text{ in}^2/\text{ft}^2 \text{ deck}$ $\rightarrow \Delta_{\text{volume}}$ (AASHTO) = +38%; $\Delta_{\text{weight}} = -66 \%$ $\rightarrow \Delta_{\text{volume}}$ (FDOT) = +14%; $\Delta_{\text{weight}} = -72 \%$



- C BEAM 1

Comparison of Rebar Qty. – Bridge Deck Example A

Convention CS-RC (ASTM A615):

- Thickness = 8"
- Concrete Cover = 2"
- Flexural Depth (#5's) *d* = 5.7"
- Empirical Overhang + Link-Slab (AASHTO-BDS): 2~Bot. layers = #5's @ 13.8" (0.27 in²/ft); Top long. = #4's & #5's @ 6.7" (0.46 in²/ft); Top transv. = #4's + #5's @ 4" (0.82 in²/ft).
- Empirical-Link (FDOT): 2~Bot. layers #5's @ 12" ** (0.31 in²/ft); Top Tranv. = #5's @ 4"(0.93 in²/ft). Top. long. = #5's @ 6"(0.62 in²/ft).
- Total Rebar (AASHTO): $A_s = 1.82 \text{ in}^2/\text{ft}^2$; (FDOT): $A_s = 2.17 \text{ in}^2/\text{ft}^2$.



<u>GFRP-RC</u> (ASTM D8505-23):

- Thickness = 7" $\rightarrow \Delta_{vol.}$ = -12%; Δ_{weight} = -15%
- Concrete Cover = 1"

- 2¹/₃" CLR. (TOP

__ Ç BEAM 3R

– 2" CLR. (BOT.)

- Flexural Depth (#5's) *d* = 5.7"
- Empirical Overhang + Link-Slab (AASHTO-GSG): Bot. ^{**} transv. = #5's @ 6.5" (≥0.83% = 0.57 in²); Bot. long. = #5's @ 12" (≥0.35% = 0.31 in²); Top transv. = #5's @ 4" (0.93 in²/ft). Top long. = #5's @ 6" (0.62 in²)

• Total Rebar:
$$A_f = 2.69 in^2/ft^2 deck$$
)
 $\rightarrow \Delta_{volume}$ (AASHTO) = +48%; $\Delta_{weight} = -67\%$
 $\rightarrow \Delta_{volume}$ (FDOT) = +24%; $\Delta_{weight} = -69\%$

Convention CS-RC (ASTM A615):

- Example A (End Span Rebar)
 - Empirical (AASHTO): As= 0.49*0.90 + 0.15*1.34 + 0.28*1.54 + 0.08*1.82 = 1.22 in²/ ft²
 - Empirical (FDOT): A_s = 0.49*1.24 + 0.15*1.55 + 0.28*1.86 + 0.08*2.17 = 1.53 in²/ ft²

<u>GFRP-RC</u> (ASTM D8505-23):

- Example A (End Span Rebar)
 - Empirical (both): As= 0.49*1.50 + 0.15*1.81 + 0.28*2.12 + 0.08*2.69 = 1.82 in²/ ft²

Concrete $\rightarrow \Delta_{vol.}$ (AASHTO) = -12%; Δ_{weight} = -15% for GFRP-RC Rebar $\rightarrow \Delta_{volume}$ (AASHTO) = +49%; Δ_{weight} = -63%. Rebar $\rightarrow \Delta_{volume}$ (FDOT) = +19%; Δ_{weight} = -70%.

Comparison of Rebar Qty. – Pile Bent Cap Example





GFRP Reinforced Concrete Design for Pile Bent Caps

Steven Nolan, P.E. – State Structures Design Office



October 19, 2022

FDOT



NSERC CR®NG - GFRP Reinforced Concrete Design for Pile Bent Caps

Presentation

Comparison of Rebar Qty. – Pile Bent Cap Summary

 Comparison of different design alternates for 5-piles @ 9-ft spacing (Example 1) – Higher Modulus GFRP Rebar (Ef = 6,500 psi to 8,700 psi for future enhancements to ASTM D7957)

Rebar Location	GFRP-RC (Type 0)	GFRP-RC	GFRP-RC (Type III)
	3-ft Deep Cap	3-ft Deep Cap	3-ft Deep Cap
	(E _f = 6,500 ksi)	(<i>E_f</i> = 7,250 ksi)	(E _f = 8,700 ksi)
Bars A - Flexural Top	6 ~ #8's	7 ~ #7's	6 ~ #7's
	(A _f = 4.7 in ²)	(A _f = 4.2 in ²)	(A _f = 3.6 in ²)
Bars D & E - Flexural	8 ~ #8's	7 ~ #8's	6 ~ #8's
Bottom	(A _f = 6.3 in ²)	(A _f = 5.5 in²)	(A _f = 4.7 in ²)
Bars V3 - Shear Stirrups	4-legs #5 at 11" sp.	4-legs #5 at 13" sp.	4-legs #4 at 10" sp.
	(A _f = 1.4 in ² /ft)	(A _f = 1.1 in ² /ft)	(A _f = 1.0 in ² /ft)


Comparison of Rebar Qty. – Pile Bent Cap Summary

• Comparison of different design alternates for 5-piles @ 9-ft spacing (Example 1)

Rebar Location	GFRP-RC (Type 0) 3-ft Deep Cap	⊿ _{volume} Type 0 or (Type III)	Steel-RC 3-ft Deep Cap
Bars A - Flexural Top	6 ~ #8's (A _f = 4.7 in ²)	+80% or (+38%)	6 ~ #6's (A _s = 2.6 in ²)
Bars D & E - Flexural Bottom	8 ~ #8's (A _f = 6.3 in ²)	+50% or (+12%)	7 ~ #7's (A _s = 4.2 in ²)
Bars V3 - Shear Stirrups	4-legs #5 at 11" sp. (A _f = 1.4 in ² /ft)	+75% or (+25%)	4-legs #4 at 12" sp. (A _s = 0.8 in ²)
Shear Stirrups SECTION A-A AASFIDOTO	Bars A Bars B Bars C V1 V2 V3 V2 V1 V4 V5 Bars A Bars B Bars C Bars C V1 V2 V3 V2 V1 V4 V5		

Construction: Example Projects & Lessons Learned

Projects with Fast-Facts Sheet

- <u>4th St North over Big Island Gap</u> (D7) *In-house Design*.
- 40th Ave NE over Placido Bayou (D7)
- Arthur Drive over Lynn Haven Bayou (D3)
- Bakers Haulover Cut Bulkhead Replacement (D6)
- <u>Bimini Dr Bridge on Duck Key</u> (D6)
- Cedar Key Bulkhead Rehab (D2)
- Halls River Bridge (D7) In-house Design.
- Key West Bight Ferry Terminal Extension (D7)
- <u>NE 23rd Ave over Ibis Waterway</u> (D6)
- PortMiami Tunnel Retaining Walls (D6)
- <u>South Maydell Dr over Palm River</u> (D7)
- SR-A1A Flagler Beach Seawall (Segment 3) (D5)
- SR-A1A over <u>Myrtle Creek</u> and <u>Simpson Creek</u> (D2)
- <u>SR-5 (US-17) over Trout River</u> (D2)
- SR-5 (US 41) over Morning Star and Sunset Waterways (D1)
- <u>SR-30 over St Joe Inlet</u> (D3)
- <u>SR-45 (US 41) over North Creek</u> (D1)
- <u>SR 112/I-195 Over Westshore Waterway</u> (D5)
- <u>SR-312 over Matanzas River</u> (D2)
- SR-520 over Indian River Bulkhead Rehab (D5)
- <u>Sunshine Skyway Seawall Rehabilitation</u> (D7)
- UM Innovation Bridge
- <u>UM Fate Bridge</u>
- <u>UM I-Dock</u>
- US-1 over Cow Key Channel (D6)



Upcoming Bridge and Seawall Projects

- D2: US1/King St over San Sebastian River (437428-1)
 - D2: <u>St. Augustine A1A/Avenida Menendez Seawall replacement (428271-2)</u>
- D2: CR 357 over Shired Creek (<u>437402-1</u>)
- D3: CR30A over Western Lake (443331-1)
- D3: CR30B/Indian Lagoon (<u>441185-2</u>)
- D3: CR 372/Surf Road over Otter Creek Rise (<u>442951-1</u>)
- D4: US 1/Jupiter Federal Observation Platform (<u>428400-2</u>)
- D4: SR-A1A North Causeway Bridge Observation Platform (429936-2)
- D4: 17th St/Indian River, East End-Vero (<u>446106-2</u>)
- D4: <u>SR 5/US 1 Over Earman River (442891-1)</u> *In-house Design.*
- D5: SR-A1A Seawalls Flagler Beach & Nth Volusia Co. (<u>452443-1</u> & <u>452444-1</u>)
- D5: Barracuda Blvd New Smyrna (<u>437935-1</u>) *In-house Design*.
- D5: 5th Street over Yacht Club Cut (<u>437936-1</u>)
- D5: US1 over Pellicer Ck (<u>447118-1</u>)
- D6: FKOSH Bridge Replacement... (<u>448206-1</u> & (<u>448207-1</u>)

Future Construction: D2 Example Projects

• <u>US1/King St over San Sebastian</u> <u>River (437428-1)</u>



• <u>St. Augustine A1A/Avenida Menendez</u> <u>Seawall Replacement (428271-2)</u>



Construction: D3 Example Projects

• CR30A over Western Lake (443331-1)





FINAL SECTION



• CR30B/Indian Lagoon (441185-2)





Construction: D4 Example Projects

• 17th St/Indian River, East End-Vero (446106-2)

• SR 5/US 1 Over Earman River (442891-1) *In-house Design*



Construction: D5 Example Projects

- SR-A1A Seawalls Flagler Beach & Nth Volusia Co. (452443-1 & 452444-1)
 - Barracuda Blvd over Canal Bradano (<u>437935-1</u>) *In-house Design*



Future Construction: D6 Example Projects

 Long Key Bridge Replacement (<u>448206-1</u> & (<u>448207-1</u>)

SR 5/US 1 LONG KEY BRIDGE OVER LONG KEY CHANNEL PD&E STUDY I FPID 448206-1-22-01 | ETDM 14451

Long Key Bridge Facts



Part of the 110-mile Florida Keys National Scenic Highway / All-American Road



• Seven Mile Bridge Replacement (<u>448207-1</u>)



Structures Design Office

Phone: (850) 414-4272

Fiber-Reinforced Polymer Reinforcing (Bars & Strands)

Geo-synthetic Reinforced Soil

Integrated Bridge System

Geo-synthetic Reinforced

Soil (GRS) Wall

Contact: Steven Nolan, P

FRP RC/PC Design Innovation webpage

WELCOME TO THE STRUCTURES DESIGN OFFICE

Office Manager: Will Potter, P.E. - State Structures Design Engineer

The Structures Design Office provides design guidance and technical assistance for structural, geotechnical, mechanical and electrical issues related to structural design and construction. The Structures Design Office is 1 of 3 divisions under the **Office of Design**, along with **Roadway Design**, and the **CADD Office**.



Fiber Reinforced Polymer Reinforcing

Structures Design - Transportation Innovation Fiber Reinforced Polymer (FRP) Reinforcing Bars and Strands

Overview Usage Restrictions / Parameters Design Criteria Specifications Standards Producer Quality Control Program Projects Technology Transfer (T²) FDOT Research Contact



NO text. NO call. NOTHING

is worth losing a life over.

Steven Nolan, P.E.

State Structures Design Office Florida Department of Transportation Email: <u>steven.nolan@dot.state.fl.us</u> Office #: 850-414-4272 Website: <u>https://www.fdot.gov/design/Innovation/</u>



"Advances in concrete reinforcement"

August 8-9th, 2024. Toronto, ON.





Archival webpage: https://www.fdot.gov/structures/innovation/iwfrpcs4

"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

Michigan DOT Implementation of FRP Prestressing

Steven Nolan, PE.

Presented on behalf of Matt Chynoweth, RS&H (former State Bridge Engineer and AASHTO CBS T6 Chair)

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"Advances in concrete reinforcement"

1980's: CFRP Prestressing Strand

• CFCC: Developed in Japan with first prestressed bridge application in 1988

II. APPLICATIONS OF CFCC



CFCC has been used in over 300 civil engineering projects .



- 3. Ground Anchors
- 4. Other (Architectures)



II. APPLICATIONS OF CFCC

1. CONCRETE STRUCTURES (PRE-TENSIONING) Shinmiya Bridge <u>1988.10</u> in Japan

World's first PC bridge with CFRP tendon



"Advances in concrete reinforcement"

1980's: CFRP Prestressing Strand

• CFCC: Developed in Japan with first prestressed bridge application in 1988

Former Bridge after 20-year life



New Shinmiya Bridge after 23-year life





TOKYO ROPE INTERNATIONAL

"Advances in concrete reinforcement"

2000's: CFRP Prestressing Strand

• CFCC: First USA bridge application in Michigan in 2001 – Post-Tensioning . CONCRETE STRUCTURES

> (TRANSVERSE POST-TENSIONING & EXTERNAL TENDON) Bridge Street Bridge May. 2001 in Southfield, Michigan Funded by FHWA and MDOT United States's first bridge constructed using CFRP CFCC $1 \times 37 40.0\varphi$: 9.2 m × 10 tendons, CFCC 1 × 19 21.8 \varphi : 9.0 m × 6 tendons





TOKYO ROPE INTERNATIONAL

9.4 m × 7 tendons

"Advances in concrete reinforcement"

2010's: CFRP Prestressing Strand

• 2012 (Maine) & 2012-13 (Virginia)



1. CONCRETE STRUCTURES (PRESTRESSED CONCRETE PILE)NIMMO PARKWAYin Virginia2 Test Piles, 16 Piles2012&2013



24" square pile 16 strands: CFCC 1 × 7 15.2mm Spiral: CFCC U 5.7mm







TOKYO ROPE INTERNATIONAL

"Advances in concrete reinforcement"

2010's: CFRP Prestressing Strand

• 2014 (Louisiana) & 2016 (Maine)





"Advances in concrete reinforcement"

Contacts

2010's: CFRP Prestressing Strand

• AASHTO Innovation Initiatives



https://aii.transportation.org/Pages/Carbon-Fiber-Reinforced-Polymer-Strands.aspx



This lead state team has completed all planned activities and is no longer active. The information below, as created by the active lead state team, remains available for review by users seeking additional information.

7

"Advances in concrete reinforcement"

2010's: CFRP Prestressing Strand & Bars

 MDOT Project **Fast Facts**

MDOT Implementation of Carbon Fiber Technology [PDF] 2014 MAASTO Annual Meeting (July 2014)

FDOT





AASHTO Innovation Initiative



(select Carbon Fiber Reinforced Polymer Strands M-102 over Plum Creek Bridge Replacement

Replacement of an existing earth-filled arch culver with a pre-stressed spread box beam superstructure pre-stressed and reinforced with carbon fiber reinforced polymer (CFRP) strands. The beams

PROJECT PURPOSE AND NEED: In 2013, the Michigan Department of Transportation eplaced an existing earth-filled arch culvert on M-102 (8 Mile Road) with a CERP pre-stressed spread b beam structure. M-102 is a major urban r the city of Detroit, with four lanes of traffic direction. Working on this route presents : impact to regional mobility, so the decision durable, non-corrosive materials was de made to ensure that life cycle rehabilitation can be delayed for as long as possible. The box beams are pre-stressed with CFRP st contain CFRP shear stirrups and mild rein The bridge deck is also reinforced with CF reinforcement. Each bridge consists of a si span, with eight 33" deep by 48" wide, pre



M-50/US-127 BR over NS RR Bridge Replacement \rightarrow

"Advances in concrete reinforcement"

2010's: CFRP Prestressing Strand

 MDOT Supported Research & Advocacy





MDOT CFRP Considerations



"Advances in concrete reinforcement"

2010's: CFRP Prestressing Strand & Bars

• MDOT Projects and Supporting Research



S



"Advances in concrete reinforcement"

2020's: CFRP Prestressing Strand

• MDOT Continuing Supporting Research

Evaluating Long Term Capacity & Ductility of Carbon Fiber Reinforced Polymer Prestressing & Post Tensioning Strands Project Number: SPR-1690 Contract Number: 2013-0065 Z2 Status: Complete. Start Date: 10/01/2013, End Date 09/30/2019

> Influence of Revising CFCC Guaranteed Strength on Performance of CFCC Prestressed Highway Bridge Beams Subjected to Various Environmental Conditions Project Number: OR21-018 Project Manager: Steve Kahl Principal Investigator: Nabil Grace (Lawrence Technical University)

Statewide Overall Carbon Fiber Composite Cable Bridge Monitoring

Research Manager: André Clover Project Number: OR14-039

Project Manager: Steve Kahl

Principal Investigator: Nabil Grace (Lawrence Technical University)



"Advances in concrete reinforcement"

August 8-9th, 2024. Toronto, ON.





Archival webpage: https://www.fdot.gov/structures/innovation/iwfrpcs4



NORTH CAROLINA Department of Transportation



Fiber Reinforced Polymer Technology – NCDOT History and Vision

W. Cabell Garbee, II, PE NCDOT Manufactured Products Engineer August 2024

History in North Carolina

- 2005 Glass Fiber Reinforced Polymer (GFRP) Bridge Decks
- 2014 NCDOT/NCSU Research Project 2014-09: CFRP Strands in Prestressed Cored Slab Units
- 2017 Transportation Pooled Fund Research Project 5(363): *Evaluation of 0.7 inch Carbon Fiber Reinforced Polymer Pretensioning Strands in Prestressed Beams*
- 2021 Harkers Island Replacement Bridge
 - 3200 ft long, No Structural Steel Reinforcement, Uses GFRP bars and CFRP strands
- 2023 Brunswick County NC 179B over Calabash River
 - FRP Reinforced 20" Square Concrete Piles
 - FRP Reinforced End Bents

In Progress/Proposed in North Carolina

- 2024 Tyrrell/Dare County (Alligator River), ~3 miles long
 - FRP Reinforced Square Concrete Piles and Cylinder Concrete Piles
 - FRP Reinforced Deck (supplier proposal)
- 2028 New Hanover County (Wrightsville Beach 3 Bridges)
 - FRP Reinforced Superstructure and Substructure
- TBD New Hanover County (Cape Fear River Memorial Bridge Replacement)
 - FRP Reinforced Substructure

Brunswick County Bridge Project

Beach Drive SW (NC 179 Business over Calabash River)
Structure: 576 Feet

Two 12' lanes with 4' Shoulder Northbound and 10' Multi-Use Path Southbound Replaces bridge built in 1975

Let May 2023

Piles: 20" Square
CFRP Spiral
CFRP Strand

Pile Caps:

GFRP Bar



Alligator River Bridge Project

Replace Lindsay C. Warren Bridge, US64 over Alligator River in Tyrrell and Dare Counties built in 1960 with Swing Span over Intercoastal Waterway.

Letting: 2024 Proposed:

> Fixed span with 65' vertical navigation clearance Two, 12' lanes with 8' shoulders Length approximately three miles One of Nine FHWA MegaGrant Projects, \$110 Million Grant

Piles: 36" Square 54" Cylinder CFRP Spiral CFRP Strand

Pile Caps, Columns, Footings: GFRP Bar

GFRP Supplier Proposal to use in Bridge Deck

Cape Fear Memorial Bridge Project

Replace Existing Lift Bridge, 3000FT Long, 408FT Lift Span Carrying US 17/US 76/US 421 over Cape Fear River Opened 1969, Deck Rehabilitated in 2024

Letting: TBD Proposed:

> 135-foot-high fixed span bridge (same as existing lift span clearance) FHWA MegaGrant Projects, \$242 Million Grant

Wrightsville Beach Bridge Project

Replace 2 Bridges Carrying US 74 & 1 Bridge Carrying US 76 over Banks Channel

US 74 Bridges Built in 1957 & US 76 Bridge Built in 1972

Letting: 2028 Proposed:

> Structure Lengths 100', 630', 860' Two 12' lanes with 6' Shoulder and 12' Multi-Use Path each bridge

Proposed in North Carolina

Discussion (Future Bridge Projects) Deck Reinforcement western counties/areas of heavy salt use coastal areas with seawater exposure Substructure Reinforcement coastal areas with seawater exposure

Discussion (Roadway Projects) Continuously Reinforced Concrete Pavement (demonstration/research project) Barrier

slipformed median barrier areas of heavy salt use

Harkers Island Bridge Project Vision

- Cast-in-place Concrete (Superstructure & Substructure)
 - Glass Fiber Reinforced Polymer (GFRP) Bars
- Prestressed Concrete Girders
 - Carbon Fiber Reinforced Polymer (CFRP) Strands
 - GFRP Stirrup Option (utilized)
 - CFRP Stirrup Option
- Prestressed Concrete Piles
 - CFRP Strands
 - CFRP Spiral
- "NO" Steel Reinforcement
 - Steel ONLY in the Railing (MASH compliance)

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Fiber Reinforced Polymer Technology – NCDOT History and Vision

Harkers Island



Harkers Island, NC



No in-water, seabottom disturbance, work allowed from April 1 through September 30

Prime Contractor: Balfour Beatty Infrastructure Inc

Prestress Concrete: Coastal Precast Systems Inc

Availability: August 30, 2021 Completion: October 28, 2025

ncdot.gov

Fiber Reinforced Polymer Technology – NCDOT History and Vision

Bridge No. 96

- Built 1970
- Superstructure Replacement
 2013
- Functionally Obsolete

Bridge No. 73

- Built 1969
- Posted SV 24, TTST 37
- Structurally Deficient





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Fiber Reinforced Polymer Technology – NCDOT History and Vision



Proposed Structure: 3,200'-0" 28 Spans

Harkers Island Bridge Project Quantities

- CFRP Prestressing Strand:
 - Girders: 650,000 Linear Feet
 - Piles: 325,000 Linear Feet
- GFRP Reinforcement:
 - Superstructure: 715,000 Linear Feet
 - Substructure: 200,000 Linear Feet


Harkers Island Project Material Approvals:

- 1. NCDOT Product Evaluation (Approved Product List)
- 2. NCDOT Vendor Approval (Tokyo Rope, Owens Corning-Mateenbar) Producer Facility Audit (FRP Institute for Civil Infrastructure)
- 3. Project Acceptance Testing (NCSU)









Pile production and shipment at Coastal Precast Systems Chesapeake, VA







Production of 54" FIB at Coastal Precast Systems Wilmington, NC

ncdot.gov



March 31, 2022. Mid-channel. End of phase one piles.



November 9, 2022 Glass Fiber Reinforced Polymer (GFRP) Bars



ncdot.gov

November 9, 2022 Piles, Caps, Beams...



April 27, 2023



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November 9, 2022 Piles, Caps, Columns, Beams...



November 2023





Fiber Reinforced Polymer Technology – NCDOT History and Vision

Questions?

W. Cabell Garbee, II, PE NCDOT Manufactured Products Engineer 1801 Blue Ridge Road, Raleigh, NC 27607 cgarbee@ncdot.gov



FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR CONCRETE STRUCTURES

"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

Use of FRP Reinforcing in Bridges in Oregon

Tanarat Potisuk

Oregon Department of Transportation

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ODOT Bridge Design Manual

• BDM 1.5.5.16

Table 1.5.5.16-1 Material Properties for Design of GFRP Reinforced Concrete

Material Property	Material Value
Ultimate tensile strength (ksi)	90 ksi (#4 - #6) 75 ksi (#7 - #10)
Tensile modulus of elasticity (ksi)	6,500
Ultimate tensile strain (inch/inch)	0.011

GFRP reinforcement is recommended for structural members that are located in corrosive environment and do not require high ductility. Service limit states often control design over strength limit states. Use GFRP reinforcement only in the following structural members:

- Bridge deck.
- Sound walls.
- Seawalls.
- Bridge Approach Systems.

Concerns

- Ductility for seismic event and vehicular impact (Extreme Event I and II)
- Abrasion resistance



ODOT Bridge Design Manual





Bridges with GFRP Bars





Bridges with GFRP Bars

• Millport Slough, Hwy 9 (built 2009)





Bridges with GFRP Bars

• Millport Slough, Hwy 9 (built 2009)





NBI Category

Category	NBI #	Rating
Deck Condition	58	7 Good
Superstructure	59	8 Very Good
Substructure	60	7 Good
Channel	61	8 Protected
Culvert/Retaining Walls	62	N N/A (NBI)







Bridges with GFRP Bars

• Lewis and Clark River (Rehab in 2014)

FDOT



Bridges with GFRP Bars

• Lewis and Clark River (Rehab in 2014)





Bridges with GFRP Bars

• Lewis and Clark River (Rehab in 2014)











Bridges with GFRP Bars

• Lewis and Clark River (Rehab in 2014)

05/07/2024

NBI Category

Category	NBI #	Rating
Deck Condition	58	6 Satisfactory
Superstructure	59	5 Fair
Substructure	60	4 Poor
Channel	61	7 Minor Damage
Culvert/Retaining Walls	62	N N/A (NBI)





Bridges with GFRP Bars

• Newberg Dundee Bypass – Sound Wall (2014)









Bridges with GFRP Bars



Bridges with GFRP Bars

• Butte Creek (bid and construction in 2025)



Bridges with GFRP Bars

Year	Year Structure		Quantity	
		Size	ft	
2009	Millport Slough	#5	53,500	
		#6	121,500	
2014	Lewis & Clark River	#5	33,135	
		#6	57,326	
2014	N-D Bypass Sound Wall	#4	40,436	
		#5	109,085	
2025	Butte Creek	#5	24,200	
		#6	18,500	



Bridges with GFRP Bars

Year	Structure	Bar Size	GFRP	Black Bar		Stainless steel	
			\$/ft	\$/lb	\$/ft	\$/lb	\$/ft
2009	Millport Slough	#5	1.12	1.1	1.15	4.37	4.56
		#6	1.48	1.1	1.65	4.37	6.56
2014	Lewis & Clark River	#5	1.36	NA	NA	4.54	4.74
		#6	1.66	NA	NA	4.54	6.82
2014	N-D Bypass Sound Wall	#4	1.76	0.84	0.56	NA	NA
		#5	1.53	0.84	0.88	NA	NA



Project Special Provision

- Performance specifications using ASTM D7957
- Will be made standard construction specifications soon
- Will add ASTM D8505 in ODOT BDM and standard construction specifications



Thank you



FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR CONCRETE STRUCTURES

"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

Use of FRP Reinforcing in Poland



Prof. dr hab. eng. Renata Kotynia

Lodz University of Technology

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Content:

- **1. FRP Materials**
- 2. Design standards
- 3. PhD thesis
- 4. Field applications



To replace traditional steel reinforcement **Fiber Reinforced Polymer (FRP) bars** are made of continuous fibres impregnated with polymeric resins in the pultrusion process in which the fibres are pulled and impregnated before curing in a heated die.

Bond behaviour between FRP bars and concrete depends on:

- a type of bars (GFRP, AFRP, BFRP, CFRP)
- the surface preparation
- bars geometry (shape and diameter)
- mechanical interaction, chemical adhesion and friction
- temperature
- Iong-term loading
- degradation of concrete

Surface preparation:

- sand covering
- ribbing with fiber braid

NSERC U FDOT

- spirally wound ribs
- ribs cutting in the GFRP bar (Schöck ComBAR)



FiReP International AG



ComRebars sp. z o.o.



Schöck ComBAR®



3

FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR CONCRETE STRUCTURES

"Advances in concrete reinforcement"

Comparison of FRP and steel reinforcement

 $E_{s} / E_{f} = 4 - 3 \text{ (GFRP)}$ $E_{s} / E_{f} = 3 - 2 \text{ (AFRP)}$ $E_{s} / E_{f} = 2 - 1 \text{ (CFRP)}$ $f_{f} / f_{sy} = 2.0 \text{ (GFRP)}$ $f_{f} / f_{sy} = 5.0 \text{ (AFRP)}$ $f_{f} / f_{sy} = 6.0 \text{ (CFRP)}$ $\epsilon_{su} / \epsilon_{fu} = 8 - 3 \text{ (GFRP)}$ $\epsilon_{su} / \epsilon_{fu} = 6 - 4 \text{ (AFRP)}$ $\epsilon_{su} / \epsilon_{fu} = 20 - 6 \text{ (CFRP)}$



GFRP AFRP Property Steel CFRP Longitudinal modulus, (GPa) 200 35 to 60 100 to 580 40 to 125 Longitudinal tensile strength, (MPa) 450 to 700 450 to 1600 600 to 3500 1000 to 2500 Longitudinal tensile strain, (%) 5 to 20 1.2 to 3.7 0.5 to 1.7 1.9 to 4.4



Advantages of FRP reinforcement:

- Highly resistant to chloride and chemical attack
- Applications:
- structures in marine environments
- in the ground
- in the chemical and industrial plants
- in thin structural concrete elements
- tensile strength is higher than that of steel
- weight is **four times lower** than that of steel
- transparent to magnetic fields and radar frequencies
- high cuttability in temporary applications (recommended to temporary reinforced concrete structures – diaphragm walls, which have to be partially, destroyed by TBM machine)



Disadvantages of FRP reinforcement:

GFRP bars:

- low tensile modulus comparing to steel
- sensitivity to abrasion from handling
- sensitivity alkalise
- relatively low fatigue resistance

CFRP bars:

- high cost
- high brittleness
- electrical conductivity

AFRP bars:

- hygroscopic absorption of moisture up to 10% of fiber at high moisture
- tend to crack internally at pre-existing micro-voids with longitudinal splitting
- low compressive strength
- loss of strength and modulus at elevated temperatures
- difficulty in cutting and machining
- sensitivity to UV lights



FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR CONCRETE STRUCTURES

"Advances in concrete reinforcement"

Design guidelines: FprEN 1992-1-1:2023(E) Annex R

Annex R (informative)

Embedded FRP reinforcement

R.1 Use of this annex

(1) This Informative Annex contains supplementary guidance for the design of new structures reinforced with non-prestressed glass and carbon fibre reinforcement.

NOTE National choice on the application of this Informative Annex is given in the National Annex. If the National Annex contains no information on the application of this Informative Annex, it can be used.

R.2 Scope and field of application

(1) This Informative Annex applies only to profiled or roughened glass and carbon fibre reinforced polymer (FRP) reinforcement bars and mesh. Prestressed FRP is not covered.

(2) This Informative Annex does not apply for use of FRP reinforcement in lightweight concrete and in concrete made with recycled aggregate, i.e. Annexes M and N do not apply.

(3) This Informative Annex applies to members with FRP reinforcement subjected to predominantly static loads, i.e. Clause 10 and Annex E do not apply to members with FRP reinforcement.

R.3 General

(1) The provisions of this Eurocode apply for concrete members with FRP reinforcement unless modified in this Annex R.

NOTE The provisions of this Eurocode apply for concrete members with FRP reinforcement unless modified in this Annex R .

R.4 Verification- Partial factors for FRP reinforcement

(1) The partial factor for material γ_{FRP} shall be used for FRP reinforcement.

NOTE The values of the partial factors for FRP reinforcement are those in Table R.1 (NDP) unless the National Annex gives different values.

Table R.1 (NDP) -	- Partial factors for	FRP reinforcement
-------------------	-----------------------	-------------------

Design Situation	γfrp
Persistent and transient design situation	1,50
Accidental design situation	1,10
Serviceability limit state	1,00



Design guidelines - Flexure



For steel reinforcement, strength to stiffness ratio is similar to normal concrete - neutral axis depth for a balanced rectangular section is around the middle of the overall effective depth.

For FRP reinforcement, the strength-to-stiffness ratio is much greater than that of concrete - the neutral axis depth for the balanced section is very close to the compressive edge.


"Advances in concrete reinforcement"

Bending moment redistribution



"Advances in concrete reinforcement"

Bending moment redistribution



Annex R - Embedded reinforcement FprEN 1992-1-1:2022 (E)

Annex R provides design rules for member reinforced with embedded FRP reinforcement within the following limits of applicability:

- Minimum modulus of elasticity of: $E_{fR} \ge 40$ GPa
- Maximum FRP strain: $\varepsilon_{fk} = \frac{f_{fk,100a}}{E_{fR}} \ge 0.005$
- Characteristic compressive strength of concrete: $f_{ck} \ge 20 \text{ MPa}$
- Maximum degree of reinforcement: $\rho_{lf} \leq 0.05$
- γ_{FRP} FRP factor

Design Situation	Ŷfrp
Ultimate Limit States (Persistent and transient design situation)	1.50
Accidental Actions	1.10
Serviceability	1.10



Annex R - Embedded reinforcement FprEN 1992-1-1:2022 (E)

R.5.3 Design assumptions





 f_{fk0} – short term tensile strength $f_{fk.100a}$ – long term tensile strength E_{fR} – elasticity modulus of FRP reinforcement γ_{FRP} – FRP factor

 C_t - factor considering temperature effects. The following values may be used for C_t = 1.0 for indoor and underground environments C_t = 0.8 for outdoor members if heating through solar radiation cannot be excluded

 C_c - coefficient between the strength under sustained load and the strength under short-term load equal 0.35 for GFRP and 0.8 for CFRP.

```
C_e - coefficient of outdoor; C_e = 0.7
```

"Advances in concrete reinforcement"

LODZ UNIVERSITY OF TECHNOLOGY Faculty of Civil Engineering, Architecture and Environmental Engineering PHD THESIS



ANALYSIS OF THE EFFECT OF PRIMARY REINFORCEMENT ON THE SHEAR RESISTANCE OF CONCRETE BEAMS WITHOUT TRANSVERSE REINFORCEMENT

Monika Kaszubska, M.Sc.D. Eng.

Renata Kotynia, Prof. of PŁ - Promoter Prof. Joaquim António Oliveira de Barros Łódź, 2018



Shear in FRP / steel reinforced beams without stirrups

Top reinforcement- GFRP bars #10 and #6 (at spacing 210mm)





Shear in FRP / steel reinforced beams without stirrups



The GFRP reinforced beams indicated gradual and slow diagonal cracking development, while failure of the steel reinforced beams was sudden and brittle.



"Advances in concrete reinforcement"





Example of failure mode

Shear tension

Flexular cracks through the full depth of beam

Small cracks – bond loss of bottom reinforcement

Flexural cracks for two reinforcement layers in GFRP reinforced beams and reverse situation for steel reinforced beams

"Advances in concrete reinforcement"

LODZ UNIVERSITY OF TECHNOLOGY **Faculty of Civil Engineering, Architecture and Environmental Engineering** PHD THESIS



ANALYSIS OF THE TYPE REINFORCEMENT INFLUENCE ON THE SHEAR CAPACITY OF CONCRETE BEAMS WITH TRANSVERSE REINFORCEMENT

Damian Szczech, M.Sc.D. Eng.

Renata Kotynia, Prof. of PŁ - Promoter Łódź. 2024



"Advances in concrete reinforcement"

Shear in GFRP / steel reinforced beams with stirrups

VARIABLE PARAMETERS										
	No.	Element	Cross section	Type of reinf.	Flexural reinf.	Rein. ratio ρ _l [%]	Stirrup diameter	Stirrups spacing	ρ _w [%]	f _{ck} [MPa]
	1.	T-G-525-8/250-30	т	-	5 # 25	2,91	8	250 mm	0,16	30
	2.	T-G-525-8/200-30	Т		5 # 25	2,91	8	200 mm	0,20	30
	3.	T-G-525-8/120-30	Т		5 # 25	2,91	8	120 mm	0,33	30
	4.	T-G-525-12/270-30	Т		5 # 25	2,91	12	270 mm	0,33	30
	5.	T-G-525-N-30	Т	GERD	5 # 25	2,91	NO	NO STIRRUPS		30
	6.	T-G-528-8/250-30	Т	GFRP	5 # 28	3,69	8	250 mm	0,16	30
	7.	T-G-528-8/200-30	Т		5 # 28	3,69	8	200 mm	0,20	30
_	8.	T-G-528-8/120-30	Т		5 # 28	3,69	8	120 mm	0,33	30
SERIES	9.	T-G-528-12/270-30	Т		5 # 28	3,69	12	270 mm	0,33	30
	10.	T-G-528-N-30	Т		5 # 28	3,69	NO	NO STIRRUPS		30
	11.	T-S-525-8/260-30	Т	STEEL	5 # 25	2,91	8	260 mm	0,15	30
	12.	T-S-525-8/200-30	Т		5 # 25	2,91	8	200 mm	0,20	30
	13.	T-S-525-8/110-30	Т		5 # 25	2,91	8	110 mm	0,37	30
	14.	T-S-525-N-30	Т		5 # 25	2,91	NO) STIRRUPS		30
	15.	T-S-528-8/260-30	Т		5 # 28	3,69	8	260 mm	0,15	30
	16.	T-S-528-8/200-30	Т		5 # 28	3,69	8	200 mm	0,20	30
	17.	T-S-528-8/110-30	Т		5 # 28	3,69	8	110 mm	0,37	30
	18.	T-S-528-N-30	Т		5 # 28	3,69	NO STIRRUPS			30



Shear in GFRP / steel reinforced beams with stirrups



"Advances in concrete reinforcement"

GFRP reinforced beams

Steel reinforced beams



Field Applications - Poland

Contractors: Mostostal Warszawa S.A., Promost Consulting T. Siwowski S.J., Rzeszow University of Technology

- Location: "Com-bridge innovative road bridge FRP" in Błażowa
- FRP Producer: ComRebars Sp. z o.

GFRP reinforced bridge decks over the Ryjak River – road no. 1411R in Błażowa





Field Applications - Poland

Contractors:Mostostal Warszawa S.A., Budmost Rafał Jędrzejek, SHM KrakówLocation:Pedestrian and bicycle bridge across the Kamienica River in Nowy SączFRP Producer:ComRebars Sp. z o.

Epsilon GFRP bars with optical sensors for structural monitoring





Field Applications - Poland

- Contractors: Mostostal Warszawa S.A.,
- Location: Fountain Park in Warsaw

BFRP reinforced concrete fountain foundation slab





Field Applications - Poland

- Contractors: Mostostal Warszawa S.A.
- Location: Provincial Hospital for Nervous and Mentally Ill in Drewnica (2014)
- FRP Producer: Mostostal Warszawa S.A.

GFRP rod reinforcement under the tank for storage of 809 m³ of water (seasonal thermal energy storage from a solar installation).







Field Applications - Poland

Contractors: Skanska S.A.

Location: Institute of Aviation, Warsaw

FRP Producer: ComRebars Sp. z o.

Application of GFRP rods for reinforcement of an industrial floor in a complex of technical buildings







Field Applications - Poland

Contractors: Skanska S.A.

Location: Reconstruction of the cross street in Lublin

FRP Producer: ComRebars Sp. z o.







Field Applications - Poland

Contractors:Budimex S.A.Location:Metro in WarsawFRP Producer:FiReP, Switzerland









"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

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August 8-9, 2024 - Toronto, Ontario

Use of FRP Reinforcing in Saudi Arabia

Ehab Ahmed and Ali Alibrahim

Alternative Rebar Plant (ARP)

Dammam; Kingdom of Saudi Arabia

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Outline

The key points in this presentation include:

- Saudi Market for FRP An Overview
- Current Specs and Codes
- GFRP-RC Projects Lessons Learned and Impact
- Local Manufacturing and Supply
- Alternative Rebar Plant (ARP)
 - Most Recent GFRP Manufacturing Facility Inaugurated by Aramco in November 2023





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Saudi Market for FRP – An Overview

FRP market in Saudi Arabia has been growing through the last decade. This is attributed to:

- Support for composites industry from Aramco.
- أرامكو السعودية saudi aramco NES
- Design of infrastructure projects for 75-100 years.
- Service / Exposure conditions.
- Saudi Industrial Development Fund (SIDF).
- Kingdom's 2030 Vision.



The economic transformation under Vision 2030 is propelling Saudi Arabia towards a future of diversity and prosperity. Through strategic investments, the fostering of new industries, and the nurturing of exceptional leadership, the Kingdom is accelerating growth and building a resilient economy that stands at the forefront of global innovation and development.

باماً YEARS



صندوق التنمية الصناعية السعودى

4

Saudi Market for FRP – An Overview

FRP market in Saudi Arabia has been growing through the last decade. This is attributed to:

- Experience gained from mega projects designed with GFRP such as the Jizan flood mitigation channel.
- Life cycle cost analysis of the projects designed with GFRP against that designed with epoxy coated steel (ECS) supported the use of GFRP.
- Availability of FRP manufactures and suppliers.

• ACI Saudi Arabian Chapter – Eastern Province (KFUPM).

Saudi Market for FRP – An Overview

FRP market in Saudi Arabia has been growing through the last decade. This is attributed to:

 Availability of FRP Specs and Codes (Aramco, SASO, ACI 440.1R, and AASHTO GFRP 2008)



Materials System Specification

12-SAMSS-027	10 December 2017
Fiber-Reinforced Polymer Bar Materials	for Concrete Reinforcement

Document Responsibility: Non-metallic Standards Committee

Note: ACI 440.6-08 E = 5700 ksi (39.3 GPa)



المملكـــة العربيــــة السعوديــــة KINGDOM OF SAUDIA ARABIA

الهيئة السعودية للمواصفات والمقايريس والجرودة Saudi Standards, Metrology and Quality Org.(SASO)

andard Specification		مواصفة قيا <mark>س</mark> ية
Standard No.	SASO-GSO-2488	رقم المواصفة
Reference No.	GSO-2488	مرجع المواصفة
Issue Year	2017	سنة الاصدار
Adoption Year	2017	سنة الاعتماد
Numeric Coding	950-2488	الترميز الرقمى

مواصفة الأسداخ من البوليمر المقوى بالألياف الزجاجية لتسليح الخرسانة

Specification for fiber-reinforced polymers (FRP): FRP (Bars, Grids and sheet)

Note: CSA S807-10 E = 40, 50, 60 GPa



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Current Specs and Standards

Updated Specs and Design Codes:

ARAMCO Material Specification (2021) – Based on ASTM D7957



Materials System Specification

24 February 2021

12-SAMSS-027 Standard Specification for Solid Round Glass Fiber Reinforced Polymer Bars for Concrete Reinforcement

Document Responsibility: Nonmetallic Standards Committee



Designation: D7957/D7957M - 22

Standard Specification for Solid Round Glass Fiber Reinforced Polymer Bars for Concrete Reinforcement¹

This standard is issued under the fixed designation D7957/D7957W; the number immediately following the designation indicates the year of original adoption or, in the case of revision. Be year of lest revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (c) indicates an editorial change since the last revision or reapproval.

This international standard was developed in accordance with internationally recognized principles on standardization established in the Decision on Principles for the Development of International Standards, Guides and Recommendations issued by the World Trade Organization Technical Barriers to Trade (TBT) Committee.

1. Scope

1.1 This specification covers glass fiber reinforced polymer (GFRP) bars, provided in cut lengths and bent shapes and having an external surface enhancement for concrete reinforcement. Bars covered by this specification shall meet the requirements for geometric, material, mechanical, and physical properties described herein.

1.2 Bars produced according to this standard are qualified using the test methods and must meet the requirements given by Table 1. Quality control and certification of production lots of bars are completed using the test methods and must meet the requirements given in Table 2.

1.9 The same of this consideration and an ended

system shall be used independently of the other, and values from the two systems shall not be combined.

1.7 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety, health, and environmental practices and determine the applicability of regulatory limitations prior to use.

1.8 This international standard was developed in accordance with internationally recognized principles on standardization established in the Decision on Principles for the Development of International Standards, Guides and Recommendations issued by the World Trade Organization Technical Barriers to Trade (TBT) Committee.

ASTM D7957 "45 GPa" – Straight and Bent Bars



Current Specs and Standards

Updated Specs and Design Codes:

• GSO Material Specification (2017) – Based on CSA S807-10

 هیئة التقییس لدول مجلس التعاون لدول الخلیج العربیة

 GCC STANDARDIZATION ORGANIZATION (GSO)

 هیئة التقییس دول مجلس التعاون لدول الخلیج العربیة

 Specification for fibre-reinforced polymers (FRP): FRP (Bars, Grids and sheet)

CSA S807 "40, 50, and 60 GPa" – Straight and Bent Bars



Current Specs and Standards

Updated Specs and Design Codes:

VISIONA

• Saudi Codes are being updated.





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GFRP-RC Projects – Lessons Learned and Impact

- Experience from projects designed with GFRP:
 - Jizan flood mitigation channel (Salan et al., 2021)
 - SPARK Bridge (Bader et al., 2023)

- Salan et al. 2021 "A Monumental Flood Mitigation Channel in Saudi Arabia The 21 km long lining is the world's largest concrete structure reinforced with GFRP bars" ACI Concrete International.
- Bader et al., 2023 "The Heart of Innovation in SPARK, Saudi Arabia The first bridge deck reinforced with GFRP bars in the GCC" ACI Concrete International.



Jizan Flood Mitigation Channel

- Original design was made with ECS for a 50-year service life, during which minor maintenance would be needed.
- In line with Aramco strategic decision to use nonmetallic reinforcement in concrete structures in company facilities, the design was changed to GFRP with expected maintenance-free service life of a 100 years.





Jizan Flood Mitigation Channel







Jizan Flood Mitigation Channel

• Cost analysis

Table 3:

Cost comparison for ECS and GFRP options based on a $30 \times 30 \times 0.2$ m slab panel

Expenditure item	ECS bars, \$	GFRP bars, \$	GFRP cost / ECS cost, %		
Reinforcing bars	9235	8222	89		
Concepto	17 514	15.940	9A		

ECS and GFRP had the same price per linear meter. However, <u>additional</u> 17% was required for customs and value-added taxes for the GFRP bars, as they were imported from Dubai, China, and Russia. These taxes raised the average price of GFRP bars.



SPARK Bridge Deck

• The first bridge deck reinforced with GFRP bars in the GCC.





Fig. 3: Cross sections: (a) bridge deck; and (b) approach slab (from Reference 10)

• Life cycle cost analysis showed savings of just over 314,462 USD are anticipated with the GFRP option in the 100-year period.


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Local Manufacturing and Supply

Now, there are local manufacturing and supply in Saudi Arabi and more projects are expected due to:

- The support and the vision for durable and sustainable design of infrastructure projects with extended service life.
- Availability of local raw materials.
- Updated specs and design codes (ASTM D7957; ASTM D8505; CSA S806; ACI 440.11; AASHTO GFRP 2nd Ed.; CSA S6).





S806-12

Design and construction of building structures with fibre-reinforced polymers

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Local Manufacturing and Supply

• Alternative Rebar Plant (ARP) started production in 2023. It is a 100% Saudi Made GFRP manufacturing facility.





Local Manufacturing and Supply

- ARP established under an agreement with a Canadian Technology Supplier - Tuf-Bar Inc. - to keep up the Kingdom's 2030 localization vision.
- ARP has the capacity to run GFRP bars of sizes from #2 (6 mm) to #12 (38.1 mm).







Local Manufacturing and Supply

• ARP products meet the ASTM D7957; ASTM D8505; CSA S807.





Local Manufacturing and Supply

• ARP is ISO Certified and runs full in-house quality control. ARP was inaugurated by Aramco in November 2023.





Local Manufacturing and Supply

• Recently, ARP joined the NSERC Industrial Research Chair (Dr. Brahim Benmokrane) at the University of Sherbrooke.



Thank You!



FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR CONCRETE STRUCTURES

"Advances in concrete reinforcement"

August 8-9, 2024 - Toronto, Ontario

FRP-SEAWATER SEA-SAND CONCRETE **STRUCTURES**

Tao Yu

The Hong Kong Polytechnic University

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Seawater sea-sand concrete (SSC) + FRP





Teng Jin-Guang, Yu Tao, Dai Jian-Guo, Chen Guang-Ming, 2011



A Major Research Project

Theme-based Research Project "Sustainable Marine Infrastructure Enabled by the Innovative Use of Seawater Sea-Sand Concrete and Fibre-Reinforced Polymer Composites" (Grant value: around HK\$52.4 million; Project coordinator: Prof. YU Tao)



Innovative Forms for FRP-SSC Structures





FRP-SSC beams

FRP-SSC Slabs



Tests of innovative beams



Innovative Forms for FRP-SSC Structures

Innovative FRP-SSC beams



Test set-up







Innovative Forms for FRP-SSC Structures



Beam-column connections







Innovative Forms for FRP-SSC Structures

Innovative FRP elements



Hybrid bars



FRP grid tubes



FRP stirrups



FRP Connectors





Innovative Forms for FRP-SSC Structures

FRP grid tube as a confining device







Innovative Forms for FRP-SSC Structures

FRP grid tube as shear reinforcement





Field Exposure Tests

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Station I: HZM Bridge Exposure Station







- **Commencement: 2020, 2021**
- Exposure duration: 8, 12, 24, 37, 48 months

Station II: Zhanjiang Exposure Station

- Commencement: 2022, 2023
- Exposure duration: 13, 24 months
- Four series of specimens including FRP, SSC, novel structural elements, FRP-SSC beams and sensors.

Field Exposure Tests



10-mm GFRP Bars (after field exposure for 48 months)

10-mm GFRP Bars (after field exposure for 24 months)















Field Exposure Tests

*Bonded joint specimens







FOURTH INTERNATIONAL WORKSHOP ON FRP BARS FOR **CONCRETE STRUCTURES**

"Advances in concrete reinforcement"



Field Exposure Tests

Bond Strength





Field Exposure Tests

***Bond Stiffness**





Field Exposure Tests

Bond Stiffness





Field Exposure Tests

***Beam Specimens**





Field Exposure Tests

*****Beams without sustained loading















Field Exposure Tests

*****Beams with sustained loading













Exposure Station Construction Project, Demonstration Lvsi Port, Nantong, China Project Demonstration: FRP-Seawater Concrete Slab



Source: https://www.google.com/maps



The slab

Constructed in Oct 2021


Sha Tin Sewage Treatment Works Ma Liu Shui, Sha Tin, Hong Kong Demonstration: <u>FRP-SSC Paving Slab</u>

Demonstration Project

Joint efforts of Tasks 1, 2 and 3

- Optimized SSC mix
- FRP-SSC structural design
- Optical fibre sensors
- Smart aggregates



Source: zh.wikipedia.org/wiki/沙田污水處理廠



Constructed in Dec 2022



Reconstruction Project Lai Chi Chong Pier, Hong Kong Demonstration: <u>FRP-SSC Slabs</u>

Demonstration Project The slab





Design completed, expected to be constructed in late 2024



Wave Wall Construction Project Lei Yue Mun, Hong Kong Demonstration: <u>FRP-SSC Wave Wall</u>

Demonstration Project



Source: https://www.google.com/maps

<image>

Source: CEDD, Hong Kong

Design completed, expected to be constructed in late 2024



NSERC

Mock-up Frame

- Full-scale FRP-SSC Mock-up Frame
- Installation & Casting Site: Zhu Hai, China
- Construction completed







Frames for producing 4-meterlong hybrid rebars

Mock-up Frame



Winding wavy-shaped FRP tubes

Mock-up Frame

Concrete casting 25 May 2023





Mock-up Frame

Installation 31 May 2023





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Thank you for your attention!

Tao Yu, Email: tao-cee.yu@polyu.edu.hk

The Hong Kong Polytechnic University

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