# **Chapter 23**

# **Design Exceptions and Design Variations**

23.1	General		23-1
23.2	Design I	Exceptions	23-2
	23.2.1	Coordination and Conceptual Concurrence	23-2
	23.2.2	Justification and Documentation	23-3
	23.2.3	Approval and Concurrence	23-6
	23.2.4	Sealing	23-6
	23.2.5	Concurrence Review	23-6
	23.2.6	Copies and Distribution	23-7
23.3	Design \	Variations	23-8
23.4	AASHTO	O Criteria for Critical Design Elements	23-9
Tables			
Criteria	Tables C	ross Reference	23-9
Exhibits	3		
Exhibit	23-A	Sample Request Letter for Design Exception	23-19
Exhibit	23-B	Sample Request Letter for Design Variation	23-20
Exhibit	23-C	Design Exceptions Variations and Utility Excepti	

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## **Chapter 23**

## **Design Exceptions and Design Variations**

### 23.1 General

The Department's roadway design criteria and standards are contained in this volume and are usually within the desirable ranges established by AASHTO. The values given in this volume have been accepted by FHWA and govern the design process. When it becomes necessary to deviate from the Department's criteria, early documentation and approval are required. There are two documentation and approval processes, Design Exceptions and Design Variations. When the Department's criteria are met, no Design Exception nor any Design Variation is required. However, when the Department's criteria are not met, a Design Exception or Design Variation is required. This requirement applies to all entities affecting planning, design, construction, maintenance and utility placement.

To expedite the approval and/or final concurrence of these deviations, it is important that the correct processes be followed. This chapter includes specific coordination, approval, concurrence and documentation requirements for both Design Exceptions and Design Variations. In both cases, the project file should clearly document the action taken and approval given. To aid in the identification and processing of Design Exceptions and Design Variations a process flowchart, *Exhibit 23-C*, has been provided.

When design criteria are not met for utilities, the process in the current *Utility Accommodation Manual (Topic No. 710-020-001)* is to be used.

Safety improvement projects are generally developed to address specific safety problems. Only items identified under the scope of work for the safety improvement project must meet design criteria and are subject to the design exception or design variation process. Existing features within the limits of the safety improvement project that do not meet design criteria and are not being addressed as part of the project do not require a design exception or design variation.

When the Department enters into an agreement with a Utility for joint use of a utility pole, and the Department requires the location of the pole to not be in compliance with the applicable horizontal clearance requirements, the Department or Consultant Design Engineer is responsible for the design variation or exception.

## 23.2 Design Exceptions

Design Exceptions are required when neither the Department's criteria nor AASHTO's criteria can be met for any one of the following 13 Critical Design Elements, which are typically safety related issues:

1.	Design Speed	6.	Vertical Clearance	10.	Horizontal Alignment
2	Lana Widtha	7	Grados	11	Vertical Alignment

- 2. Lane Widths 7. Grades 11. Vertical Alignment
- 3. Shoulder Widths 8. Cross Slope 12. Stopping Sight Distance
- 4. Bridge Widths 9. Superelevation 13. Horizontal Clearance
- 5. Structural Capacity

## 23.2.1 Coordination and Conceptual Concurrence

In order to allow time to research alternatives and begin the analysis and documentation activities, it is critical that Design Exceptions be identified as early in the process as possible. This is preferably done during the PD&E process for major projects and the scope development process for minor projects. It is required that approval be obtained no later than the initial engineering phase.

When the need for a Design Exception has been determined, the District Design Engineer must coordinate with the State Roadway Design Engineer's Office. The appropriate Area Design Engineer will assist in obtaining conceptual concurrence and in identifying the necessary level of effort for justification and documentation.

For Design Exceptions requiring FHWA approval, the State Roadway Design Engineer's Office will coordinate with FHWA to obtain conceptual concurrence and any required documentation requested by FHWA.

Design Exceptions impacting a structure require concurrence of both the State Roadway Design Engineer and the State Structures Design Engineer. Conceptual concurrence should first be obtained from the appropriate Area Engineer in the State Structures Design Office, and decisions or special conditions documented for Design Exception support.

This coordination effort will expedite the approval and concurrence process.

### 23.2.2 Justification and Documentation

Once conceptual concurrence of the Design Exception has been coordinated and completed, the justification must be documented. The objective of the justification of Design Exceptions is to demonstrate that the impacts on the operation and safety of the facility are acceptable, compared to the impacts and added benefits of meeting the criteria.

All Design Exceptions shall include documentation sufficient to justify the request and independently evaluate the operational and safety impacts. Design Exceptions must address the following issues unless otherwise agreed upon during conceptual concurrence:

#### 1. Description

- a. Project description (general project information, typical section, begin/end milepost, county section number, etc.).
- Description of the Design Exception (specific project conditions related to Design Exception, Critical Design Element, acceptable AASHTO and Department value and proposed value for project).
- c. The compatibility of the design and operation with the adjacent sections.

### 2. <u>Operational Impacts</u>

- a. Amount and character of traffic using the facility.
- b. Effect on capacity of the deviation (proposed criteria vs. AASHTO using an acceptable capacity analysis procedure and calculate reduction for design year, level of service).

### 3. Safety Impacts

- a. Most recent five (5) year Crash History and Analysis (location, type, severity, relation to the Design Exception element). Crash locations must be identified on copies of the plans or straight line diagrams if plans are not available.
- b. Impacts associated with proposed criteria (annualized value of expected economic loss associated with crashes).

#### 4. Benefit/Cost Analysis

Calculate a benefit/cost analysis that estimates the cost effectiveness of correcting or mitigating a substandard design feature. The benefit is the expected reduction in future crash costs and the cost is the direct right of way, construction and maintenance costs associated with the design. These costs are calculated and annualized so a direct comparison of alternate designs can be made.

A benefit/cost ratio indicates the cost effectiveness of implementing a particular

design. However, the final decision is a management decision that considers all factors important to the successful implementation of the Department's mission.

- a. The key factors considered in the analysis are:
  - 1) Evaluation of crashes by type and cause,
  - 2) Estimate of crash costs,
  - 3) Selection of a crash reduction factor,
  - 4) Utilization of a discount rate of 5%,
  - 5) Estimate of construction and maintenance costs,
  - 6) Selection of life of the improvements,
  - 7) Period of time over which the benefits will be realized.
- b. Two acceptable methods for calculating a benefit/cost analysis are:
  - 1) Historical Crash Method

This method can be used for sites with a crash history. It is basically the ratio (benefit/cost) of the estimated reduction in crash costs to the estimated increase in construction and maintenance cost. The annualized conversion will show whether the estimated expenditure of funds for the benefit will exceed the direct cost, thereby lending support as to whether the improvement should be done or not.

The following table of *Highway Safety Improvement Program Guideline* (*HSIPG*) cost per crash by facility type is used for this method:

*COST/CRASH BY FACILITY TYPE					
FACILITY TYPE	DIV	DIVIDED		UNDIVIDED	
PACIEITI TIPE	URBAN	RURAL	URBAN	RURAL	
<3 Lanes	\$68,800	\$152,200	\$78,000	\$218,900	
3 Lanes	\$47,100	\$152,200	\$52,000	\$218,900	
4 Lanes	\$74,500	\$181,200	\$53,500	\$76,400	
5 Lanes	\$52,400	\$181,200	\$53,500	\$76,400	
6 Lanes	\$63,300	\$181,200	\$53,500	\$76,400	
Interstate	\$83,600	\$195,700	\$83,600	\$195,700	
Turnpike	\$99,700	\$228,800	\$99,700	\$228,800	

All State Roads Average Cost/Crash: \$83,070

<sup>\*</sup>The above values were derived from 1994, 1995, and 1996 traffic crash and injury severity data for crashes on state roads in Florida, using the formulation described in *FHWA Technical Advisory* "*Motor Vehicle Accident Costs*", *T 7570.1*, dated June 30, 1988 and updated injury costs provided in the companion *FHWA Technical Advisory*, *T 7570.2*, dated October 31, 1994.

#### 2) **ROADSIDE 5.0** computer program

This program complements the *Roadside Design Guide* dated January 1996. The program computer disks are normally furnished with the text.

This method can be used where clear zone applies. Based on the input (offsets, traffic, slopes, crash history, traffic accident severity levels, etc.) of information available to the user, the program will offer results which can be used in comparing courses of action. The current *Roadside Design Guide* and the *FHWA Technical Advisory* titled *Motor Vehicle Accident Costs* and dated October 31, 1994 provides guidance for the benefit/cost analysis.

Using this method for Department projects, the accident severity level costs to be used, noted in the *Roadside Design Guide* are revised as follows:

* Estimated Costs for Various Traffic Accident Severity Levels				
Fatal Accident	\$2,600,000			
Severe Injury Accident	\$505,000			
Moderate Injury Accident	\$165,000			
Slight Injury Accident	\$36,167			
Property Damage Only Accident Level 2	\$10,469			
Property Damage Only Accident Level 1	\$2,000			

<sup>\*</sup>The above values were derived from the *FHWA Technical Advisory* "*Motor Vehicle Accident Costs*", *T 7570.1*, dated June 30, 1988 and updated injury costs provided in the companion *FHWA Technical Advisory*, *T 7570.2*, dated October 31, 1994.

### 5. Conclusions and Recommendations

- a. The cumulative effect of other deviations from design criteria,
- b. Safety mitigating measures considered and provided,
- c. Summarize specific course of action (Include conditional requirements such as projects in the work program that will fix deficiency).

## 23.2.3 Approval and Concurrence

Design Exceptions on projects having full federal oversight (see *Chapter 24* of this volume) and involvement are recommended by the District Design Engineer for approval by the FHWA Division Administrator.

Any Design Exception that reduces vertical clearance over an interstate roadway to less than 16 feet requires FHWA to coordinate with Military Traffic Management Command (MTMC) before the District Design Engineer can recommend the Design Exception.

Any Design Exception for design speed on the FIHS system shall require concurrence from the State Highway Engineer. All other Design Exceptions require concurrence from the State Roadway Design Engineer.

Design Exceptions impacting a structure require concurrence from the State Structures Design Engineer and the State Roadway Design Engineer.

All other projects are recommended by the Responsible Professional Engineer for approval by the District Design Engineer and concurrence by the State Roadway Design Engineer.

## 23.2.4 Sealing

All Design Exceptions are to be sealed in accordance with *Chapter 19* of this volume.

### 23.2.5 Concurrence Review

After the documentation justifying the Design Exception is forwarded to the appropriate Area Design Engineer, the Design Exception will be reviewed for completeness and adherence to the requirements of this chapter.

If the Design Exception complies with all requirements, it will be signed by the appropriate engineer(s), signifying concurrence. When necessary, the Design Exception will be forwarded by the appropriate Area Design Engineer to FHWA for approval.

## 23.2.6 Copies and Distribution

One (1) original is required by the State Roadway Design Engineer's Office (appropriate Area Design Engineer). Subsequent to obtaining all appropriate signatures for concurrence the following distribution is made:

- 1. The State Roadway Design Engineer's Office will return one (1) signed original to the District Design Engineer for files.
- 2. State Roadway Design Engineer's Office will return one (1) copy of the signed original to the District Design Engineer for submission to the Engineer of Record.
- 3. State Roadway Design Engineer's Office will retain one (1) copy of the signed original.
- 4. State Structures Design Engineer's Office will retain one (1) copy of the signed original for structure related Design Exceptions.

## 23.3 Design Variations

Design Variations are required when deviations from the Department's criteria occur. However, when both AASHTO and Department criteria for any of the 13 Critical Design Elements are not met, a Design Exception will be processed in lieu of a Design Variation.

A Design Variation request must address the following items:

- 1. Design criteria versus proposed criteria.
- 2. Reason the design criteria are not appropriate.
- Justification for the proposed criteria.
- 4. Any background information which documents or justifies the request.

Requests begin with the Responsible Professional Engineer and are submitted to the District Design Engineer for approval. This approval shall be documented in the project file as per the sample request letter *Exhibit 23-B*.

Any Design Variation for design speed on the FIHS system shall require concurrence from the State Highway Engineer.

Issues impacting a structure require final concurrence from the District Structures Design Engineer for Category 1 structures or the State Structures Design Engineer for all other structures.

As with Design Exceptions, it is critical that Design Variations be identified early in the process in order to allow time to research alternatives and begin the analysis and documentation activities. This is preferably done during the PD&E process for major projects and the scope development process for minor projects. It is required that approval be obtained no later than the initial engineering phase.

All Design Variations are to be sealed in accordance with *Chapter 19* of this volume.

The District Design Engineer will retain the original and distribute one (1) signed copy to the Engineer of Record.

## 23.4 AASHTO Criteria for Critical Design Elements

As an aid to the designer, the following tables may be used as a reference for determining when a Design Exception is required based on AASHTO criteria, but are in no way intended to replace Department design criteria. The page numbers referenced are to AASHTO's *A Policy on Geometric Design of Highways and Streets 2001* and are a starting point for researching project criteria.

#### **Criteria Tables Cross Reference**

Table Number	ble Number Title	
Table 23.4.1	AASHTO Design Speed (Minimum)	23-11
Table 23.4.2	AASHTO Lane Widths (Minimum)	23-12
Table 23.4.3	AASHTO Shoulder Widths (Minimum)	23-12
<b>Table 23.4.4</b>	AASHTO Bridge Widths (Minimum)	23-13
Table 23.4.5	AASHTO Structural Capacity (Minimum Loadings)	23-14
Table 23.4.6	AASHTO Vertical Clearance (Minimum)	23-14
Table 23.4.7	AASHTO Grades (Minimum and Maximum)	23-15
Table 23.4.8	AASHTO Cross Slope (Minimum and Maximum)	23-15
Table 23.4.9	AASHTO Superelevation (Maximum)	23-16
Table 23.4.10	AASHTO Horizontal Alignment	23-16
Table 23.4.11	AASHTO Vertical Alignment	23-17
Table 23.4.12	AASHTO Stopping Sight Distance	23-17
Table 23.4.13	AASHTO Horizontal Clearance (Minimum)	23-18

**NOTE**: AASHTO's *A Policy on Geometric Design of Highways and Streets 2004* may be used instead of the 2001 edition. The 2004 edition is in substantial conformance with the criteria in the 2001 edition, so use of either edition is acceptable. The major change in the 2004 edition is a revision of the superelevation section which resulted in minor differences in superelevation rates. The AASHTO page numbers referenced in this chapter only apply to the 2001 edition and may not correspond to the correct page numbers in the 2004 edition.

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Table 23.4.1 AASHTO Design Speed (Minimum)

Type Facility	Other Factors	Design Speed (mph)	AASHTO
Freeways	Urban Rural	50 70	pg. 507
Urban Arterials	Major Other	30 30	pg. 72
Rural Arterials	Rolling terrain Level terrain	50 60	pg. 448
Urban Collectors		30	pg. 434
Rural Collectors	Level ADT < 400 ADT 400 - 2000 ADT > 2000	40 50 60	pg. 426, Exh. 6-2
	Rolling ADT < 400 ADT 400 - 2000 ADT > 2000	30 40 50	
CBD	Major or Minor	30	pg. 434
Ramps	Highway Design Speeds (mph) 30 35 40 45 50 60 65 70	15 18 20 23 25 28 30 30 30	pg. 830
Loop Ramps	150 ft. radius	25	pg. 829
Connections	Direct Semi-Direct	40 30	pg. 829

Table 23.4.2 AASHTO Lane Widths (Minimum)

Type Facility	Lane Width (feet)	AASHTO
Freeways	12	pg. 508
Rural Arterials	11	pg. 452, Exh. 7-3
Urban Arterials	10	pg. 476
Urban Collectors	10	pg. 437
Rural Collectors	10	pg. 429, Exh. 6-6
Low Speed	10	pg. 316
Residential	9	pg. 316
Auxiliary	10	pp. 316, 437
Continuous TWLTL	10	pg. 316

Table 23.4.3 AASHTO Shoulder Widths (Minimum)

Type Facility	Other Factors	Right	Median	AASHTO
		(feet)	(feet)	
Freeways	4 lanes	10	4	pg. 509
	≥ 6 lanes	10	10	pg. 509
Rural Arterial	ADT > 2000	8		pg. 452, Exh. 7-3
	ADT 400-2000	6		
	ADT < 400	4		
	Divided highway 4 lanes	8	4 paved	pg. 459
	Divided highway 6 lanes	8	8	pg. 460
Urban Arterial	Low Type	2		pg. 318
	High Type	10		pg. 318
Heavily Traveled	High Speed (≥ 50 mph)	10		pg. 318
Rural & Urban	ADT > 2000	8		pg. 429, Exh. 6-5
Collectors	ADT 1500-2000	6		
	ADT 400-1500	5		
	ADT < 400	2		

Table 23.4.4 AASHTO Bridge Widths (Minimum)

Type Facility	Other Factors	Bridge Widths	AASHTO
Freeways	New Bridges	Approach Roadway Width	pg. 510
Rural Arterials	New Bridges (Short)	Approach Roadway Width	pg. 451
Aiteriais	New Long Bridges (> 200 ft.)	Travel Lanes + 4 ft. each side	pg. 451
	Remain in Place	Travel Lanes + 2 ft. each side	pg. 451
Urban Arterials	Long (> 200 ft.), where shoulders or parking lanes are provided on the arterial	Travel Lanes + 4 ft. each side	pg. 485
	All new bridges	Curb to curb width of street	pg. 485

		Bridge Widths		
Type Facility	Other Factors	New or Reconstruction	To Remain	AASHTO
Rural and Urban	Under 400 ADT	Traveled Way + 2 ft. each side (1)	22 ft. <sup>(2)</sup>	pp. 430, 431
Collectors	ADT 400-1500	Traveled Way + 3 ft. each side (1)	22 ft. <sup>(2)</sup>	pp. 430, 431
	ADT 1500-2000	Traveled Way + 4 ft. each side <sup>(1),(3)</sup>	24 ft. <sup>(2)</sup>	pp. 430, 431
	ADT > 2000	Approach Roadway Width (1),(3)	28 ft. <sup>(2)</sup>	pp. 430, 431

<sup>1.</sup> If the approach roadway has paved shoulders, then the surfaced width shall be carried across the bridge.

<sup>2.</sup> Bridges longer than 100 ft. are to be analyzed individually.

<sup>3.</sup> For bridges > 100 ft. in length, the minimum bridge width of traveled way plus 3 ft. on each side is acceptable.

**Table 23.4.5 AASHTO Structural Capacity (Minimum Loadings)** 

Type Facility	Other Factors	Loading	AASHTO
Freeways		HS-20	pg. 510
Rural Arterials		HS-20	pg. 451
Urban Arterials		HS-20	pg. 451
Local Roads	New & Reconstruction Bridges	HS-20	pg. 390, Exh. 5-6
Local Roads	Existing Bridges	H 15	pg. 390, Exh. 5-7
Collectors	New & Reconstruction Bridges	HS-20	pg. 430, Exh. 6-6
Collectors	Existing Bridges	H 15	pg. 431, Exh. 6-7

**Table 23.4.6 AASHTO Vertical Clearance (Minimum)** 

Type Facility	Vertical Clearance (feet)	AASHTO
Freeways	16 <sup>(1),(2)</sup>	pp. 510, 511, 767, 768
Arterials: Rural Urban	16 <sup>(1),(2)</sup> 16 <sup>(1),(2)</sup>	pp. 451, 767, 768 476, 767, 768
Other Highways	14 <sup>(2)</sup>	pp. 389, 511
Sign Trusses	17 <sup>(2)</sup>	pg. 511
Pedestrian Overpass	17 <sup>(2)</sup>	pg. 511
Tunnels: Freeways Other Highways	16 <sup>(2)</sup> 14 <sup>(2)</sup>	pg. 359 pg. 359
Railroads	23 (2)	pg. 526

<sup>1. 14</sup> feet allowed in highly developed urban areas if alternate route has 16 feet.

<sup>2.</sup> Minimum value that can be used without a Design Exception. An allowance of 6 inches should be added to vertical clearance to accommodate future resurfacing.

Table 23.4.7 AASHTO Grades (Minimum and Maximum)

#### Maximum Grades

Туре	Type		Gr	ades (	%) For	Desig	n Spe	ed (mp	oh)		AASHTO
Facility	Terrain	30	35	40	45	50	55	60	65	70	AASHIO
Freeway (1)	Level					4	4	3	3	3	pg. 510,
	Rolling					5	5	4	4	4	Exh. 8-1
Rural Arterial	Level			5	5	4	4	3	3	3	pg. 450,
	Rolling			6	6	5	5	4	4	4	Exh. 7-2
Urban Arterial:	Level	8	7	7	6	6	5	5			pg. 476,
	Rolling	9	8	8	7	7	6	6			Exh. 7-10
Rural Collector <sup>(2)</sup>	Level	7	7	7	7	6	6	5			pg. 427,
	Rolling	9	9	8	8	7	7	6			Exh. 6-4
Urban Collector <sup>(2)</sup>	Level	9	9	9	8	7	7	6			pg. 436,
	Rolling	11	10	10	9	8	8	7			Exh. 6-8

- 1. Grades one percent steeper than the values shown may be used for extreme cases in urban areas where development precludes the use of flatter grades and for one-way downgrades.
- 2. Short lengths of grade in rural and urban areas, such as grades less than 500 ft. in length, one-way downgrades, and grades on low-volume rural and urban collectors may be up to 2 percent steeper than the grades shown above.

#### Minimum Grades for Urban Curb & Gutter

Type Facility	Minimum %	AASHTO
Arterials	as required for adequate drainage	pg. 475
Collector Roads & Streets	0.30	pg. 435
Local Roads & Streets	0.20	pg. 395

Table 23.4.8 AASHTO Cross Slope (Minimum and Maximum)

Type Facility	Other Factors	Minimum	Maximum	AASHTO
Freeways		0.015	0.025 <sup>(1)</sup>	pg. 508
Arterials	Rural	0.015	0.02 <sup>(1)</sup>	pg. 450
	Urban	0.015	0.03	pg. 476
Divided Highways		0.015	0.02 (1)	pg. 459
Collectors	Rural	0.015	0.02 <sup>(1)</sup>	pg. 425
	Urban	0.015	0.03	pg. 435
Shoulders	Paved	0.02	0.06	pg. 320
	Gravel	0.04	0.06	pg. 320
	Turf	0.06 <sup>(2)</sup>	0.08 <sup>(2)</sup>	pg. 320

- 1. Values given are for up to two lanes in one direction. Additional outside lanes may have cross slopes of 0.03.
- 2. Shoulder cross slopes which meet FDOT criteria do not require a Design Exception.

### **Table 23.4.9 AASHTO Superelevation (Maximum)**

Type Facility	Superelevation Rate	AASHTO
Highways (Rural)	0.12	pg. 141
Urban	0.06	pg. 142
Low Speed Urban w/severe constraints	None	pg. 142
Ramps and Turning Roadways at Intersections	0.10	pg. 643

### **Table 23.4.10 AASHTO Horizontal Alignment**

Minimum Radius (feet) with Superelevation (page 145, Exh. 3-14)

Type Facility	Super- elevation		Minimum Curve Radius (feet) for Design Speed (mph)										
	e-max	15	20	25	30	35	40	45	50	55	60	65	70
Rural	0.04	70	125	205	300	420	565	730	930	1190	1505		
Highways and High	0.06	65	115	185	275	380	510	660	835	1065	1340	1660	2050
	0.08	60	105	170	250	350	465	600	760	965	1205	1485	1820
Speed Urban	0.10	55	100	160	230	320	430	555	695	880	1095	1345	1640
Streets	0.12	50	90	145	215	300	395	510	645	810	1005	1230	1490

Minimum Radius (feet) for Section with Normal Cross Slope (page 168, Exh. 3-26)

Type		Minimum Curve Radius (feet) for Design Speed (mph)										
Facility	15	20	25	30	35	40	45	50	55	60	65	70
All	960	1700	2460	3350	4390	5570	6880	8350	9960	11720	13180	14730

Minimum Radius (feet) for Intersection Curves (page 201, Exh. 3-43)

Design Speed (MPH)	10	15	20	25	30	35	40	45
Minimum Radius (feet)	25	50	90	150	230	310	430	540
Assumed Minimum Superelevation Rate	0.02	0.02	0.02	0.04	0.06	0.08	0.09	0.10

Minimum Passing Sight Distance (feet) (page 124, Exh. 3-7)

Design Speed (mph)	20	25	30	35	40	45	50	55	60	65	70
Passing Sight Distance	710	900	1090	1280	1470	1625	1835	1985	2135	2285	2480

### **Table 23.4.11 AASHTO Vertical Alignment**

(Taken from page 426, Exh. 6-2)

Design Speed	K Value <sup>(1)</sup> for Vertical C	urves Rounded for Design
(mph)	Crest	Sag
15	3	10
20	7	17
25	12	26
30	19	37
35	29	49
40	44	64
45	61	79
50	84	96
55	114	115
60	151	136

<sup>1.</sup> Rate of vertical curvature, K, is the length of curve per percent algebraic difference in the intersecting grades.

### **Table 23.4.12 AASHTO Stopping Sight Distance**

(Taken from page 112, Exh. 3-1)

Design Speed (mph)	Stopping Sight Distance (feet) Computed for Design	
15	80	
20	115	
25	155	
30	200	
35	250	
40	305	
45	360	
50	425	
55	495	
60	570	
65	645	
70	730	

**Table 23.4.13 AASHTO Horizontal Clearance (Minimum)** 

Feature			Cle	earance	AASHTO	
Bridges			See Ta	able 23.4.4		
Tunnels			2.5 ft. f	rom edge of traffic lane	pg. 358	
Underpasses	2-lane:		Norma	l shoulder width (to edge of barrier) (1)	pg. 766, Exh. 10-6	
	Divided	Roadway:				
Barrier Wall & Guardrail			pg. 766, Exh. 10-6			
Light Poles (2)	Rural:		Outside clear zone (if non-breakaway)			
	Urban:		pg. 323			
Trees greater than 4 inches in diameter measured 6 inches above	Rural Urban:	Arterials: Collectors ≤ 45 Collectors > 45		Outside clear zone 10 ft. from traveled way Outside clear zone 1.5 ft. from face of curb	pg. 403, 485 pg. 431 pg. 431 pg. 403,441,485	
the ground	Freewa	ys (Rural and Ur	ban):	Outside clear zone	pg. 511	
Sign supports			Outside	e clear zone (if non-breakaway)	pg. 299	
Utility Poles (2)	Rural:		Outside	e clear zone	pg. 298	
	Urban:		1.5 ft. f	rom face of curb	pp. 297, 323	
Building Line			pg. 526			
Signal Pole	Rural:		pg. 4-13 <sup>(3)</sup>			
and Controller Cabinets	Urban:		1.5 ft. f	rom face of curb	pg. 323	

<sup>1.</sup> For metal guardrail, add deflection distance.

<sup>2.</sup> Exceptions for utility poles are to be in accordance with the current *Utility Accommodation Manual* exceptions procedure for horizontal clearance for utility poles.

<sup>3.</sup> AASHTO Roadside Design Guide.

### Exhibit 23-A Sample Request Letter for Design Exception

To: <sup>(1)</sup>	,	Date:		
County Section Number: Federal Aid Number: Project Description: Begin Project MP:	End Project MP: RRR I II III IV	State Road Num		
	quested for the following elemen			
	( ) Lane Widths ( ) S ( ) Vertical Clearance ( ) C ( ) Horizontal Alignment ( ) V		) Bridge V ) Cross S ) Stopping	Vidths lope g Sight Distance
	oncerning the project and items of connentation to this exhibit in accordan		uding crash	history and plans
	Prepared by <sup>(2)</sup> : Recommended by:		essional Engi	neer (print or type)
			Date	
		Consult	ant Firm (prir	nt or type)
Recommended by <sup>(2)</sup> : Approved by <sup>(3)</sup> :				
Concurrence (4):	District Design Engineer		Date	1 1
Concurrence (5):	State Roadway Design Engine			
Approved by (3):	State Structures Design Engir	neer	Date	<u> </u>
Concurrence (4):	FHWA Division Administrator		Date	<u> </u>
	State Highway Engineer			

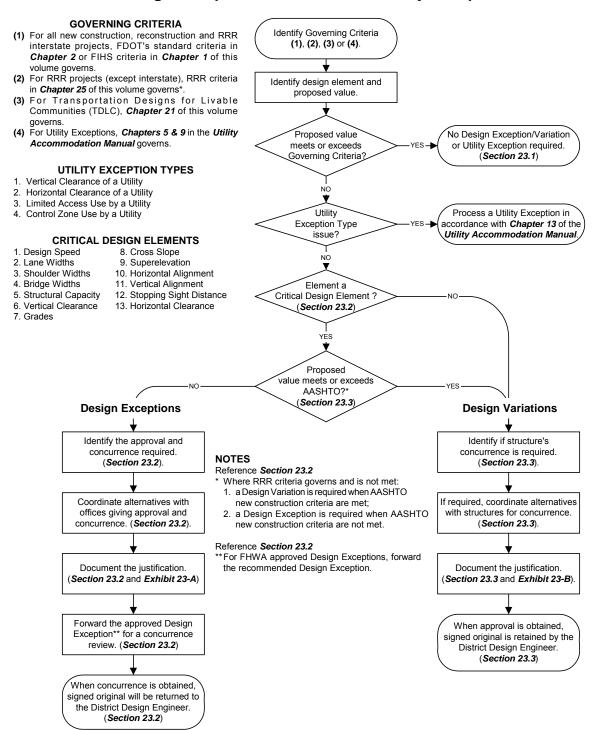
- 1. Design exceptions on projects having full federal oversight and involvement are addressed to the FHWA Division Administrator. All other design exceptions are addressed to the District Design Engineer.
- Design exceptions on projects having full federal oversight and involvement are recommended by the District Design Engineer
  and prepared by the Responsible Professional Engineer. All other design exceptions are recommended by the Responsible
  Professional Engineer.
- 3. Design exceptions on projects having full federal oversight and involvement are approved by the FHWA Division Administrator. All other design exceptions are approved by the District Design Engineer.
- 4. Design exceptions for design speed on the FIHS requires concurrence from the State Highway Engineer following a review with the State Transportation Planner. All other design exceptions require concurrence from the State Roadway Design Engineer.
- 5. Design exceptions impacting the geometry, vertical clearance, layout of structures, or superstructure cross slope require concurrence from the State Structures Design.

### **Exhibit 23-B Sample Request Letter for Design Variation**

To:	, District Desig	n Engineer Date:
Financial Project I County Section No Federal Aid Numb Project Description Begin Project MP:	esign Variation D: umber: er: er:End Project MP:RRR	Federal Oversight: Yes No
A design variation	is requested for the following element(s	):
( ) Structural Capa	(2) ( ) Lane Widths ( ) Shacity ( ) Vertical Clearance ( ) Grad ( ) Horizontal Alignment ( ) Vertical ( ) Other	
Indicate the design description of the Address all issues	Design Variation. and each of the Items listed under <b>Sect</b>	ation is being requested, along with a specific
	Recommended by: _	Responsible Professional Engineer (print or type)
	-	Date//
		Consultant Firm (print or type)
Approved by:	District Design Engineer	Date//
Concurrence (1): _	State or District Structures Design En	Date/ gineer
Concurrence (2): _	State Highway Engineer	/ Date//

- Design variations impacting the geometry, vertical clearance, layout of structures, or superstructure cross slope require concurrence from the District Structures Design Engineer for Category 1 structures and from the State Structures Design Engineer for all other structures.
- 2. Design speed variations on the FIHS requires concurrence from the State Highway Engineer following a review with the State Transportation Planner.

### Exhibit 23-C Design Exceptions Variations and Utility Exceptions Process



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