

CASE 3

(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

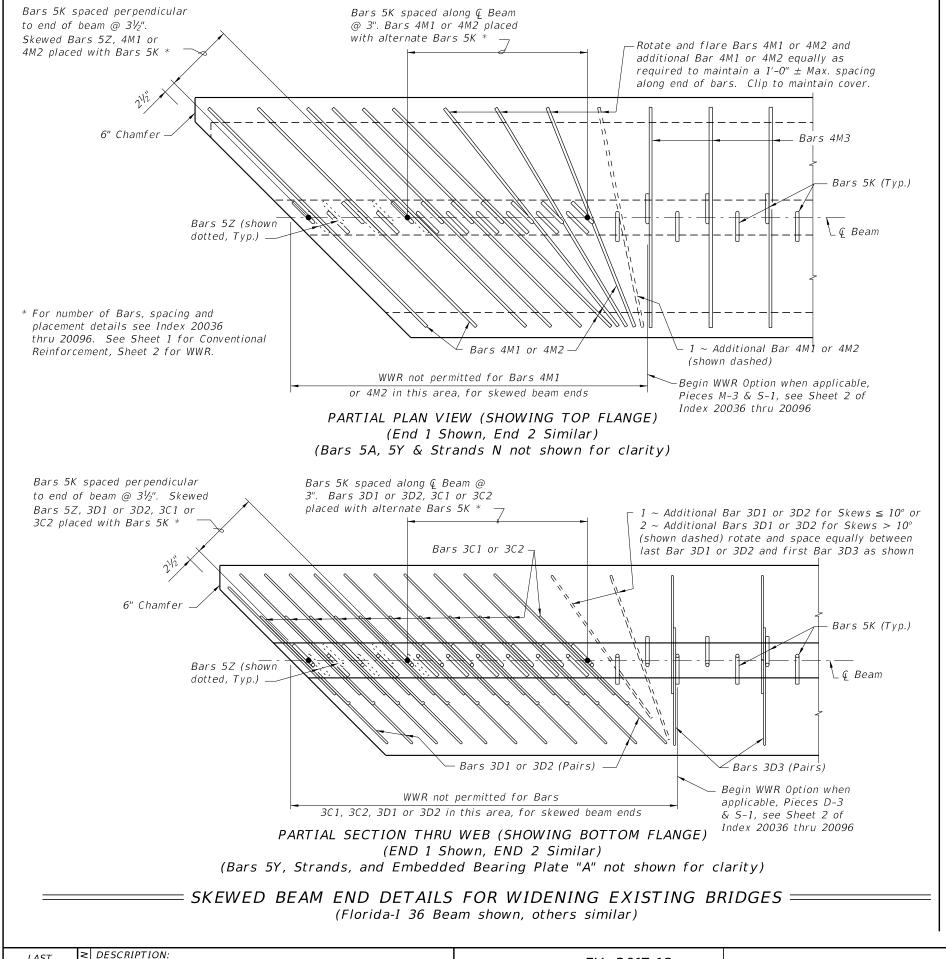
BEAM NOTES

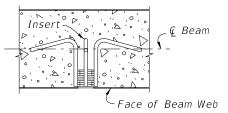
- 1. Work this Index with the Florida-I Beam Standard Details (Index 20036, 20045, 20054, 20063, 20072, 20078, 20084 and 20096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum.
- 4. Strands N: ¾" Ø minimum, stressed to 10,000 lbs. each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
  - A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands.
  - For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
  - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
  - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
  - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.
- D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement.
- 10. Contractor Options:
  - A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
  - B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Saftey Line Anchorage Devices are permitted in the top flange to accomodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.

DESCRIPTION:

CONDITION 3

SCHEMATIC END ELEVATIONS OF BEAMS (Showing Vertical Bevel of Beam End)





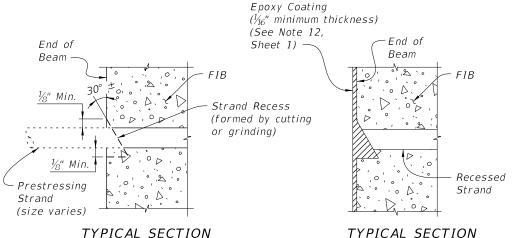
## PLAN SECTION THRU BEAM WEB AT INSERT FOR DIAPHRAGM REINFORCING

(When Intermediate Diaphragms are Required by Design)

#### **INSERT NOTES**

- 1. Provide 1" Ø, zinc-electroplated, ferrule wing nut or coil inserts, UNC threads, 1/0 minimum gage wire, not more than 4" in depth with a minimum ultimate tensile strength of 11,400 lbs. in 4,000 psi concrete.
- 2. If inserts are needed on both sides (faces) of beam webs, an assembly as long as the thickness of the beam web, consisting of two (2) ferrule or coil inserts attached by two (2) or more struts may be utilized. The connecting struts shall have a minimum ultimate tensile strength of 11,400 lbs.
- 3. Inserts for diaphragm reinforcing are required at each end of each intermediate diaphragm shown on the Beam Framing Plan and may be required at the end of the beams when end diaphragms are shown. See Superstructure and Beam Framing Plans for longitudinal location of inserts for each face of beam.

==== INSERT DETAIL ====



SHOWING CUT STRAND RECESS LIMITS

AFTER PROTECTING

==== STRAND CUTTING AND PROTECTING DETAIL ====

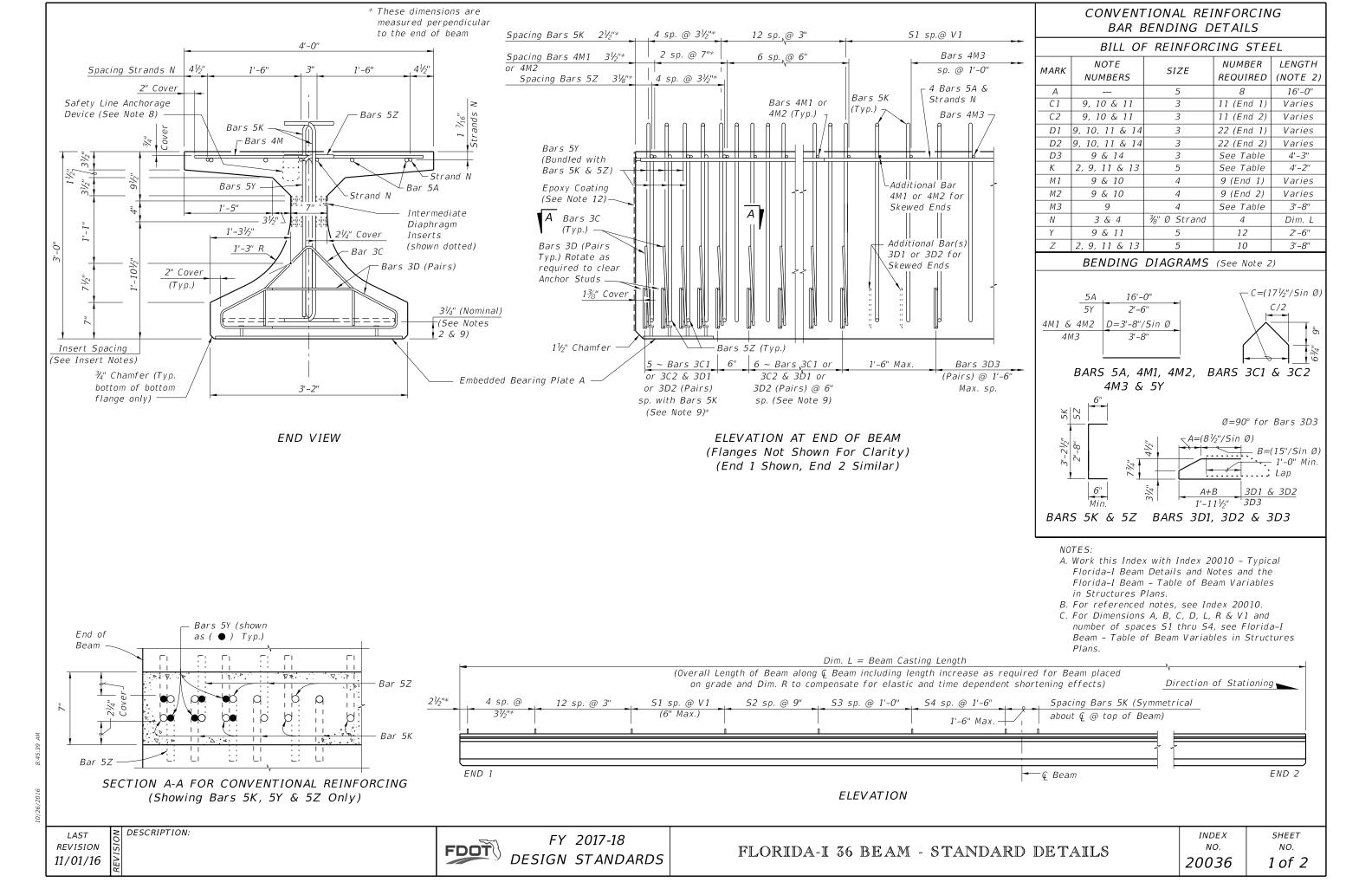
REVISION 11/01/16

FY 2017-18 **DESIGN STANDARDS** 

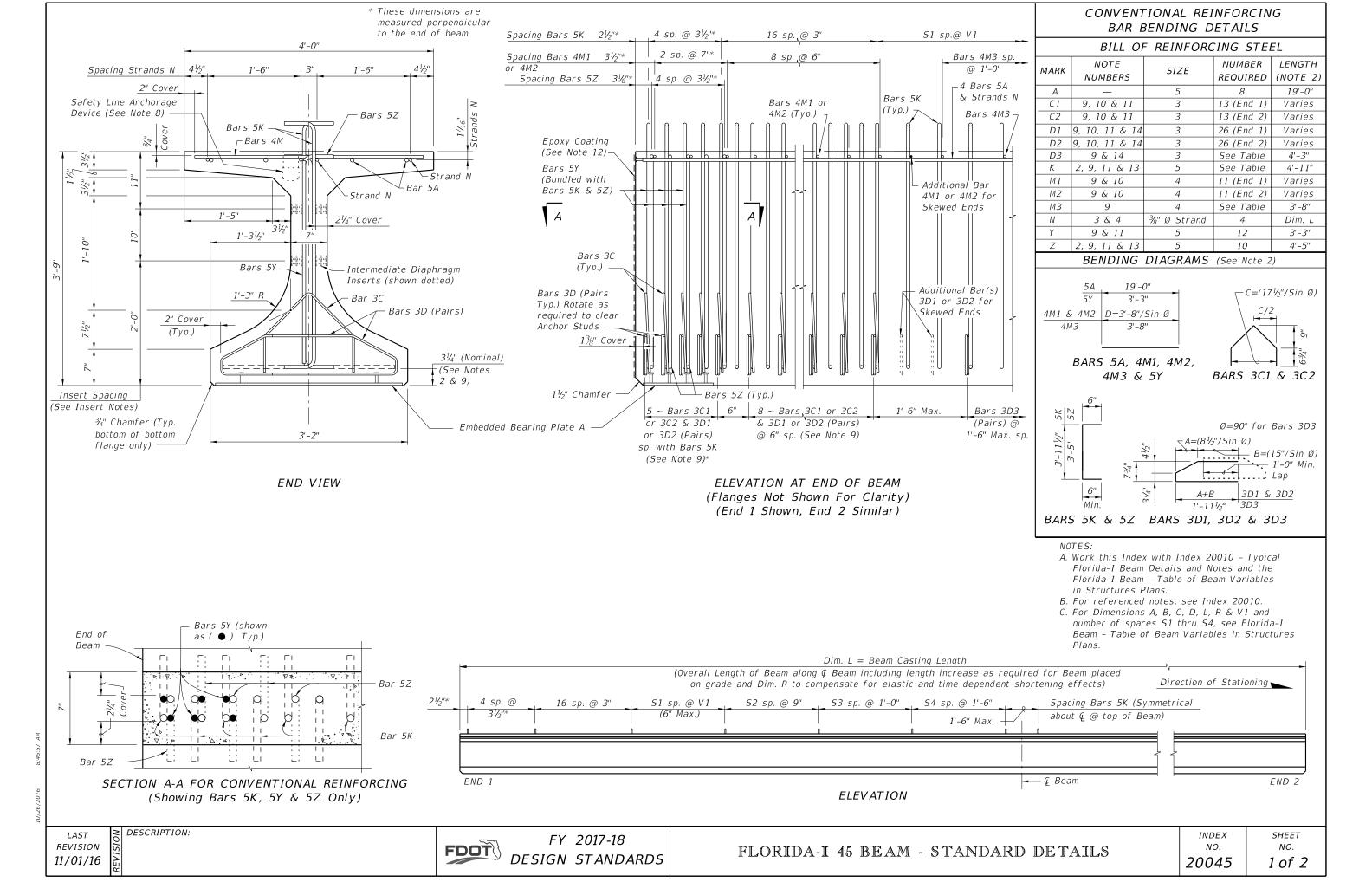
TYPICAL FLORIDA-I BEAM

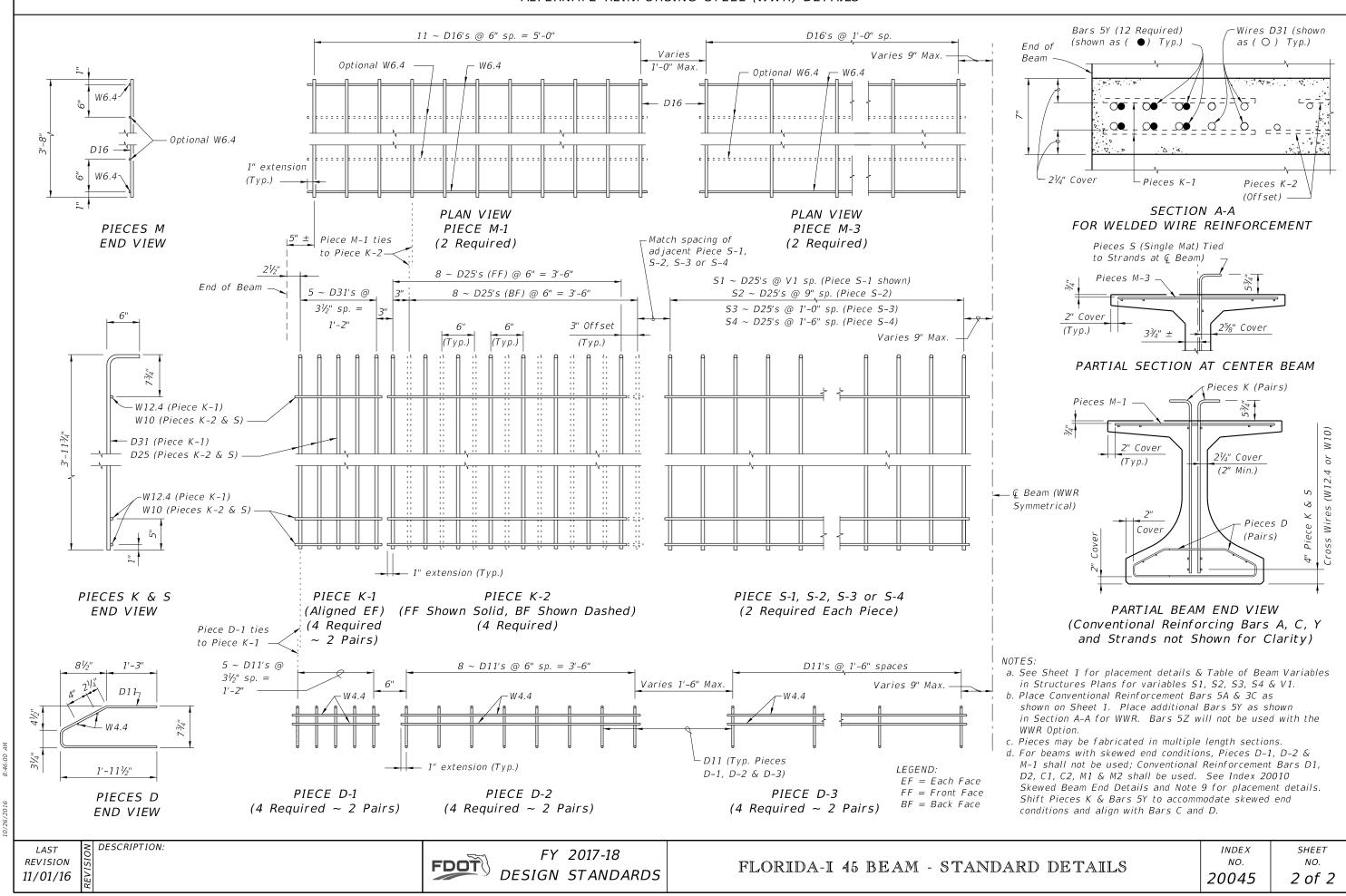
INDEX NO. 20010

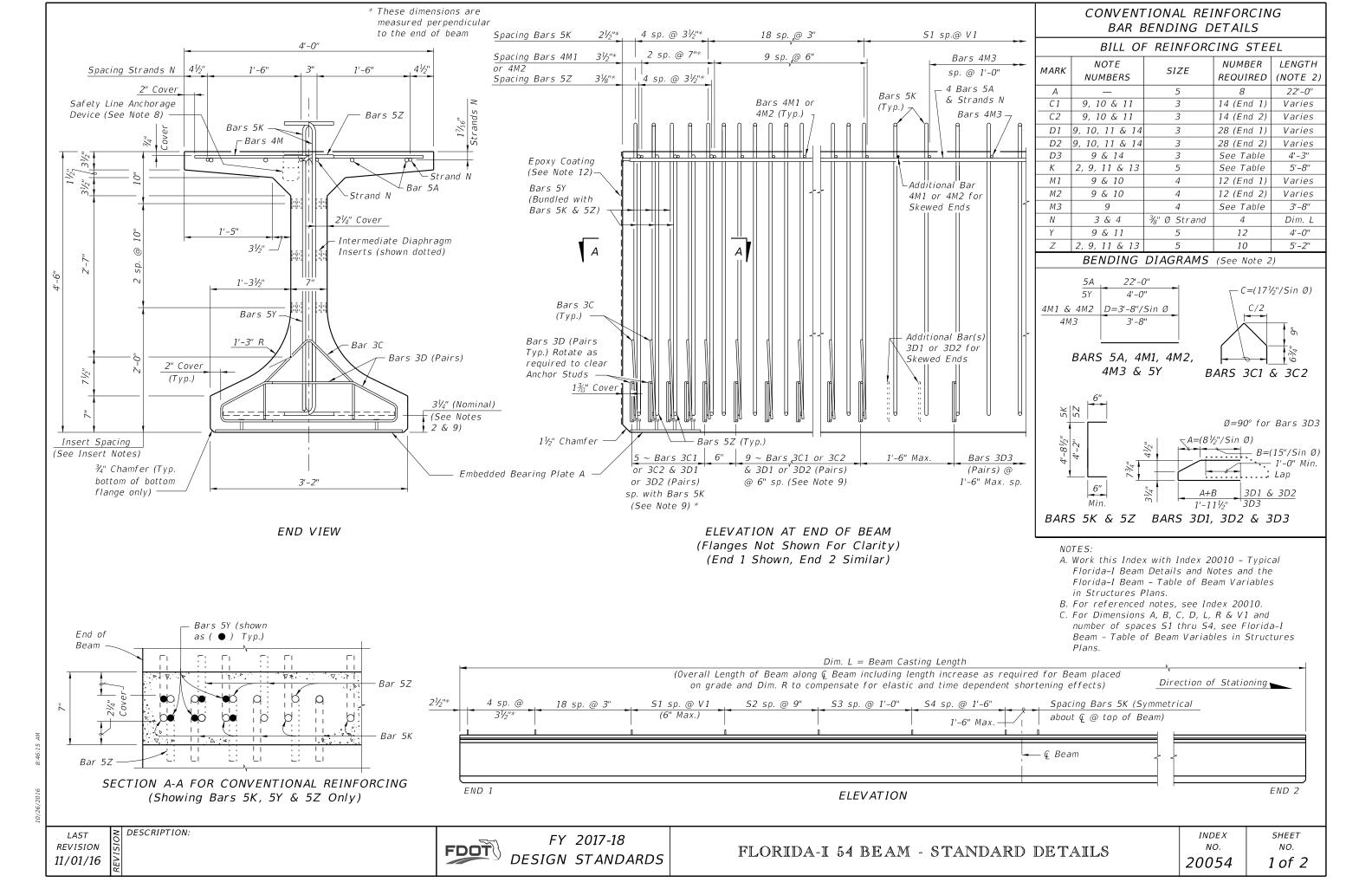
SHEET NO. 2 of 2

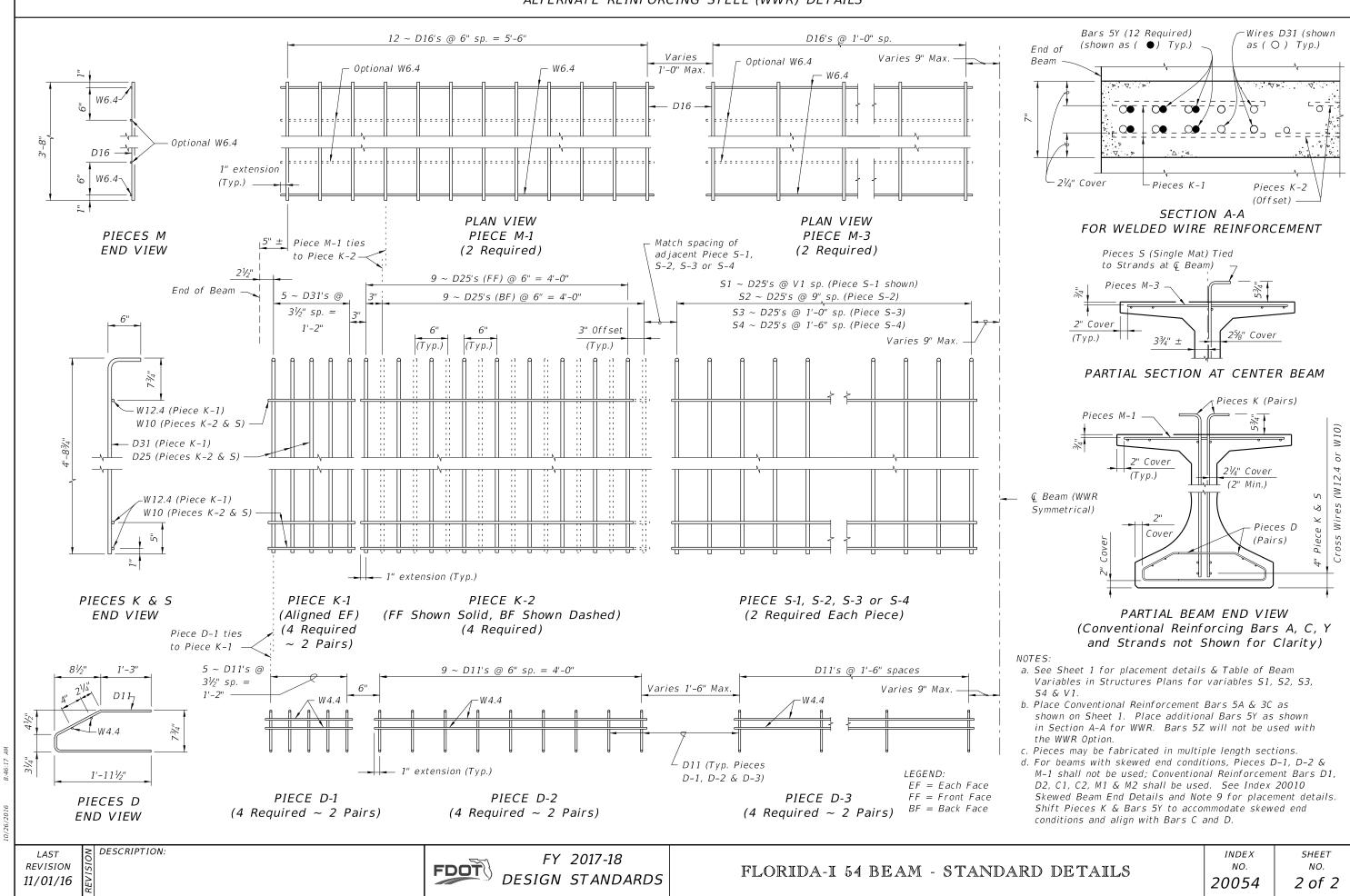


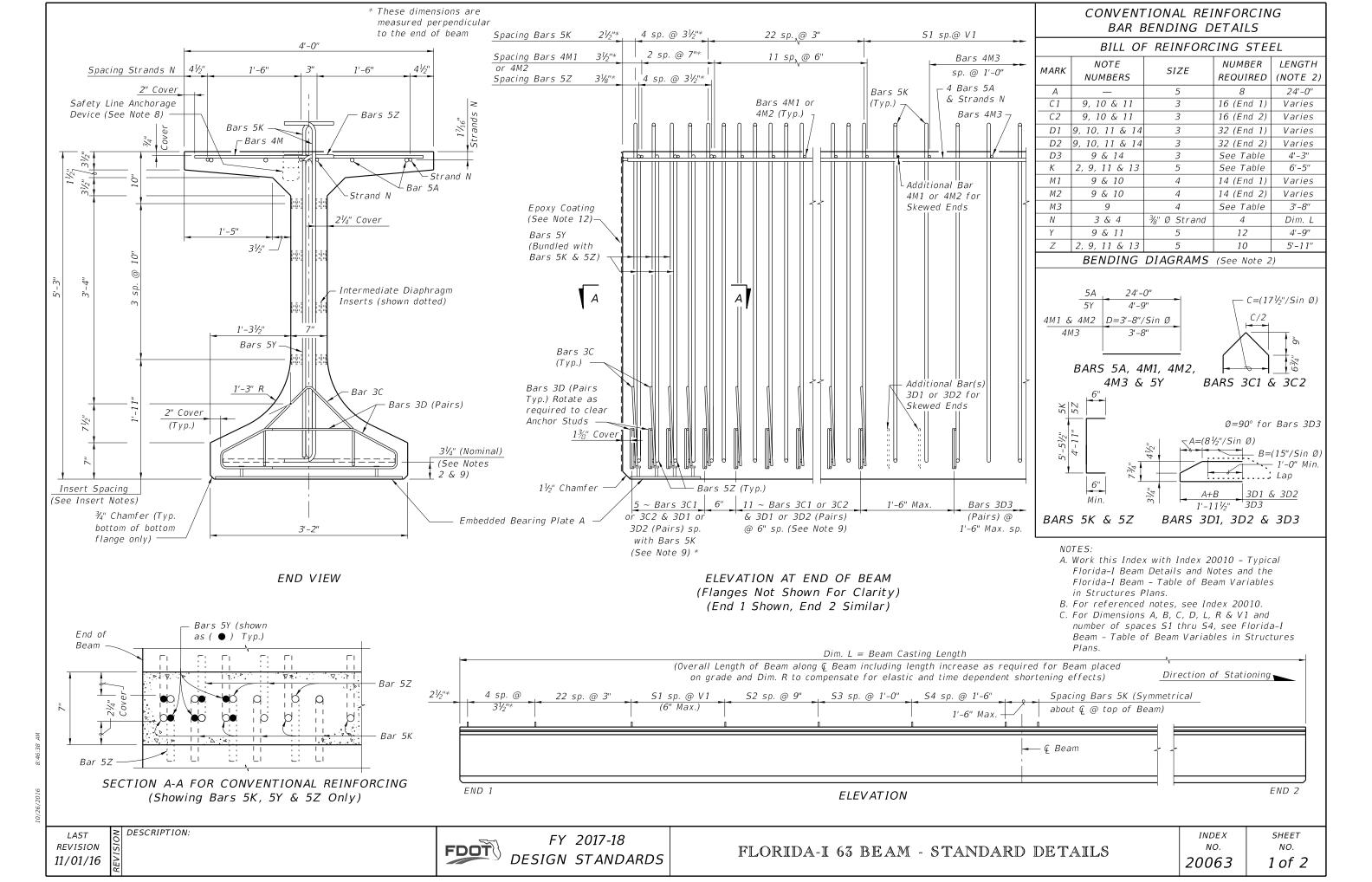
#### ALTERNATE REINFORCING STEEL (WWR) DETAILS 9 ~ D16's @ 6" sp. = 4'-0" D16's @ 1'-0" sp. Bars 5Y (12 Required) -Wires D31 (shown (shown as ( ●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max Optional W6.4 -W6.4 1'-0" Max. Beam — Optional W6.4 - W6.4 W6.4-D16-Optional W6.4 D16 1" extension W6.4 (Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) PLAN VIEW PLAN VIEW SECTION A-A PIECES M PIECE M-1 PIECE M-3 – Match spacing FOR WELDED WIRE REINFORCEMENT 5" ± Piece M-1 ties END VIEW of adjacent (2 Required) (2 Required) to Piece K-2 Piece S-1, S-2, Pieces S (Single Mat) Tied 6 ~ D25's (FF) S-3 or S-4 21/5" to Strands at © Beam) S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam — Pieces M-3 6 ~ D25's (BF) S2 ~ D25's @ 9" sp. (Piece S-2) @ 6'' = 2'-6'' $3\frac{1}{2}$ " sp. = S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 2" Cover Varies 9" Max. 25/8" Cover (Typ.)(Typ.) (Typ.)(Typ.)3¾" ± PARTIAL SECTION AT CENTER BEAM W12.4 (Piece K-1) Pieces K (Pairs) W10 (Pieces K-2 & S) Pieces M-1 W10) or D31 (Piece K-1 (W12.4 2" Cover D25 (Pieces K-2 & S) -21/4" Cover (Typ.)(2" Min.) Ø Wires ( .\_\_Ç Beam (WWR -W12.4 (Piece K-1) Symmetrical) Pieces D W10 (Pieces K-2 & S) Piece Cover (Pairs) 1" extension (Typ.) PARTIAL BEAM END VIEW PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 (Conventional Reinforcing Bars A, C, Y END VIEW (Aligned EF) (FF Shown Solid, (2 Required Each Piece) and Strands not Shown for Clarity) (4 Required ~ BF Shown Dashed) Piece D-1 ties to Piece K-1 2 Pairs) (4 Required) NOTES: a. See Sheet 1 for placement details & Table of Beam 5 ~ D11's @ $6 \sim D11's @ 6'' sp. = 2'-6''$ D11's @ 1'-6" spaces 1'-3" Variables in Structures Plans for variables S1, S2, S3, $3\frac{1}{2}$ " sp. = Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, - 1" extension (Typ.) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index 20010 D-1, D-2 & D-3) EF = Each FaceSkewed Beam End Details and Note 9 for placement details. PIECE D-1 PIECE D-2 PIECE D-3 FF = Front Face Shift Pieces K & Bars 5Y to accommodate skewed end PIECES D BF = Back Faceconditions and align with Bars C and D. (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) **END VIEW** DESCRIPTION: INDEX SHEET FY 2017-18 REVISION NO. NO. FDOT FLORIDA-I 36 BEAM - STANDARD DETAILS **DESIGN STANDARDS** 11/01/16 20036 2 of 2

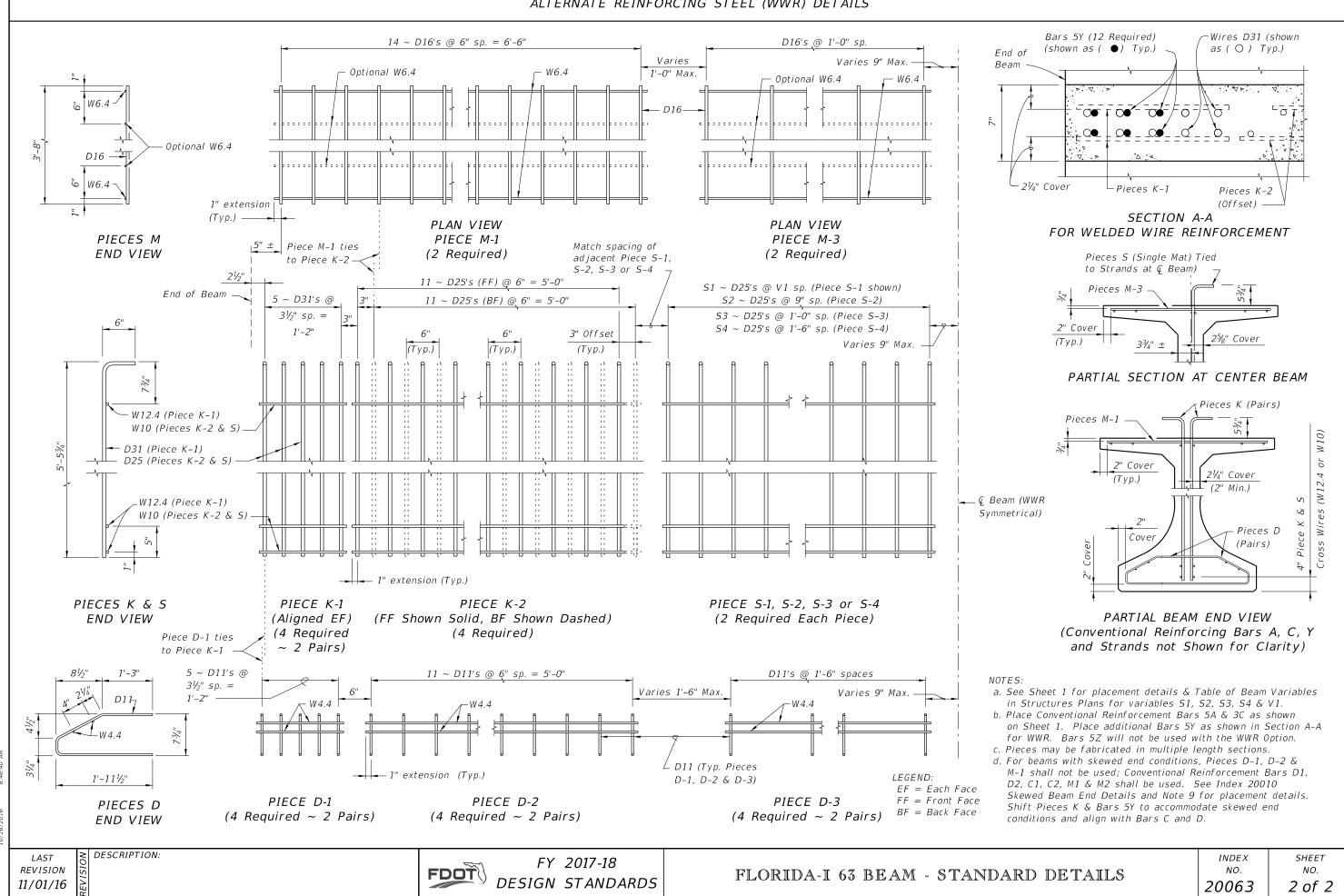


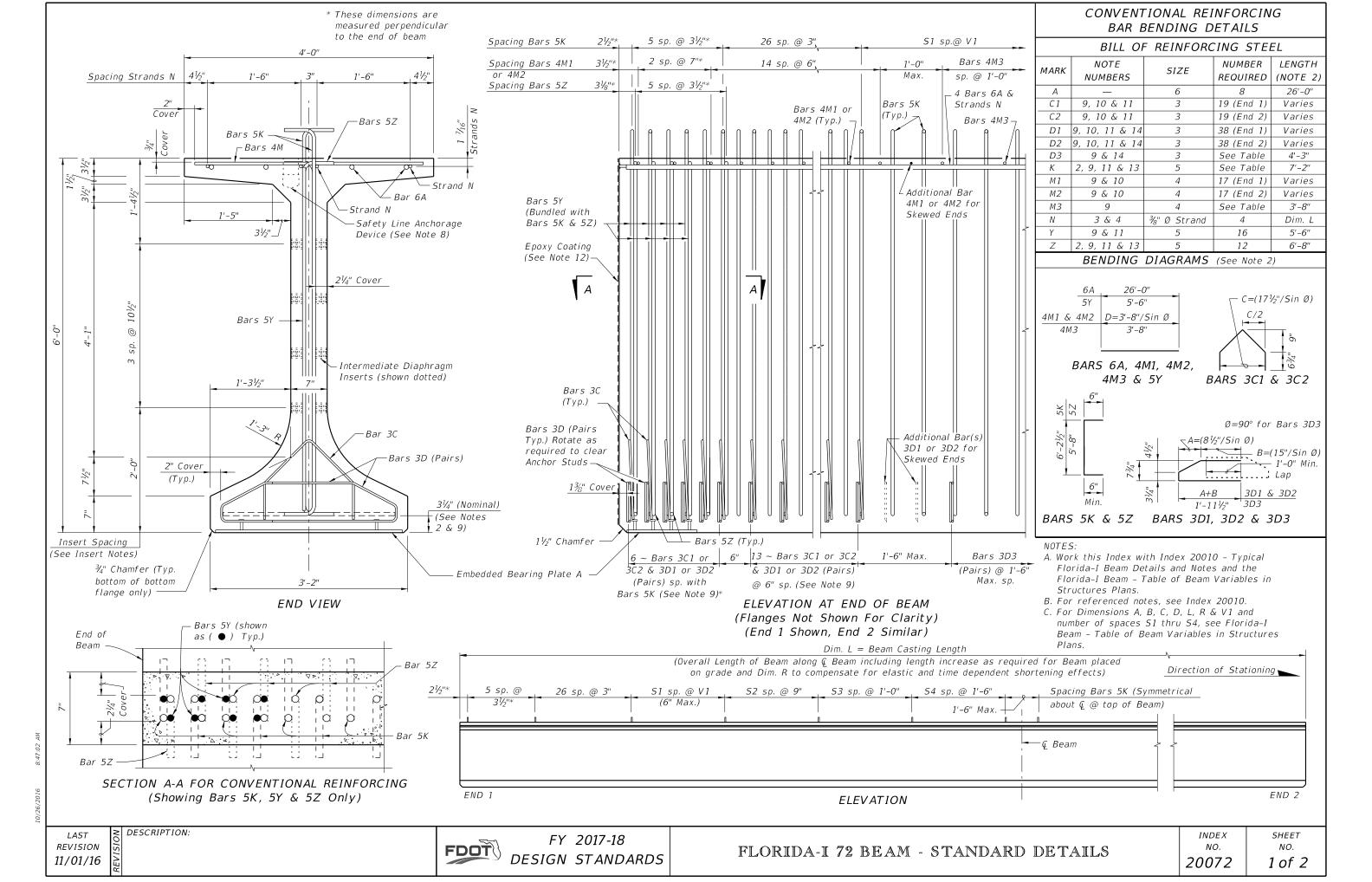


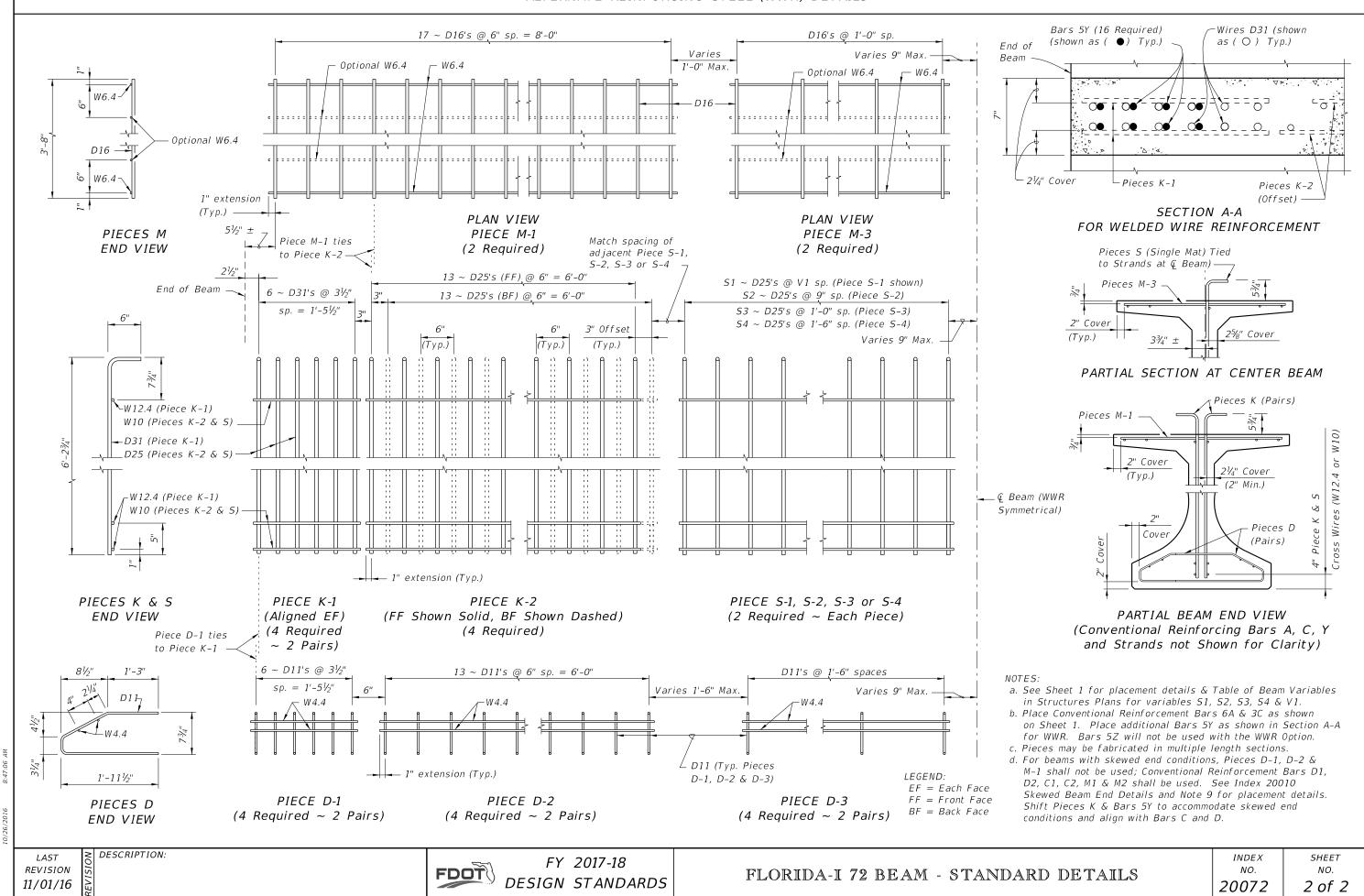


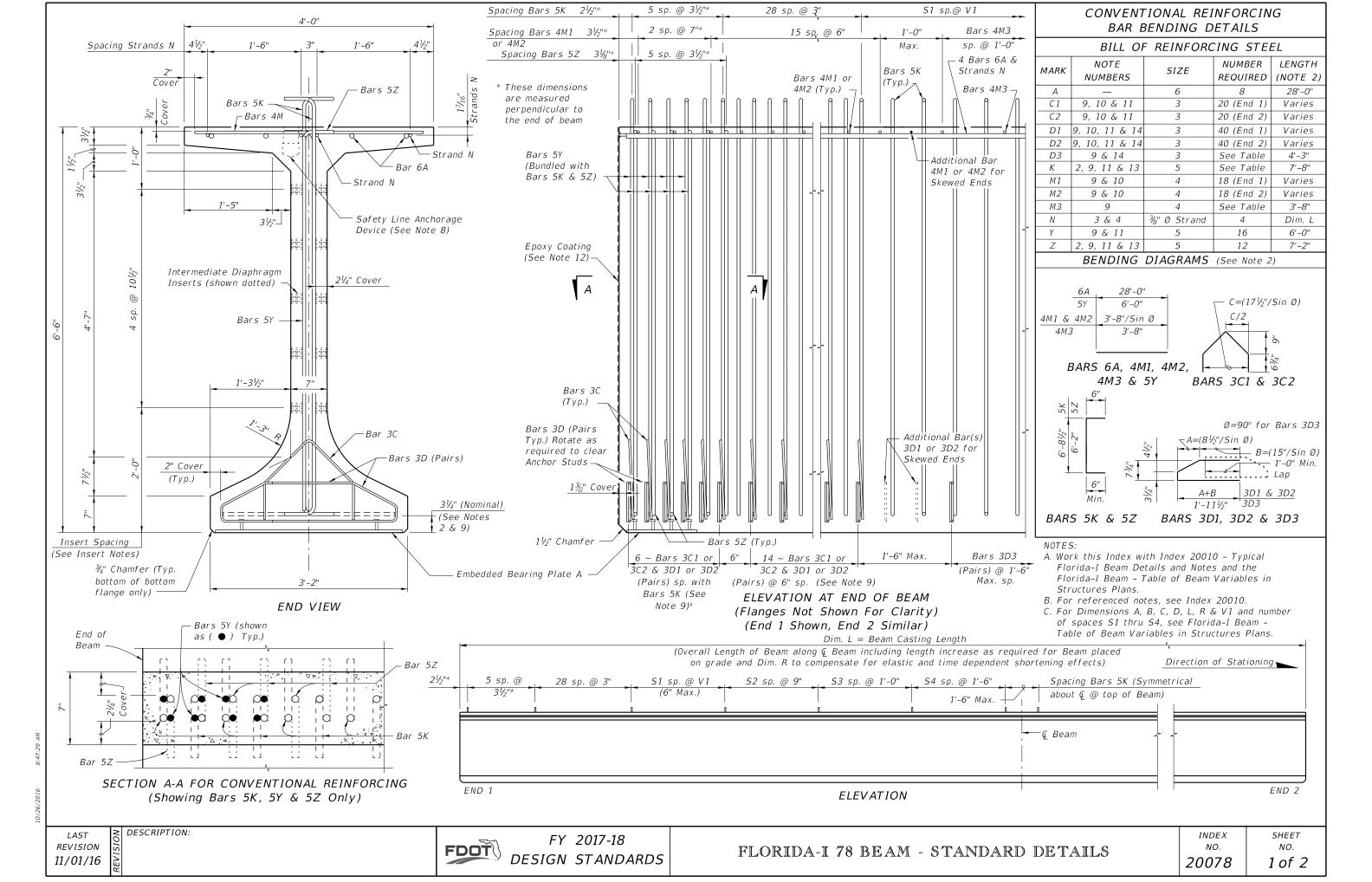


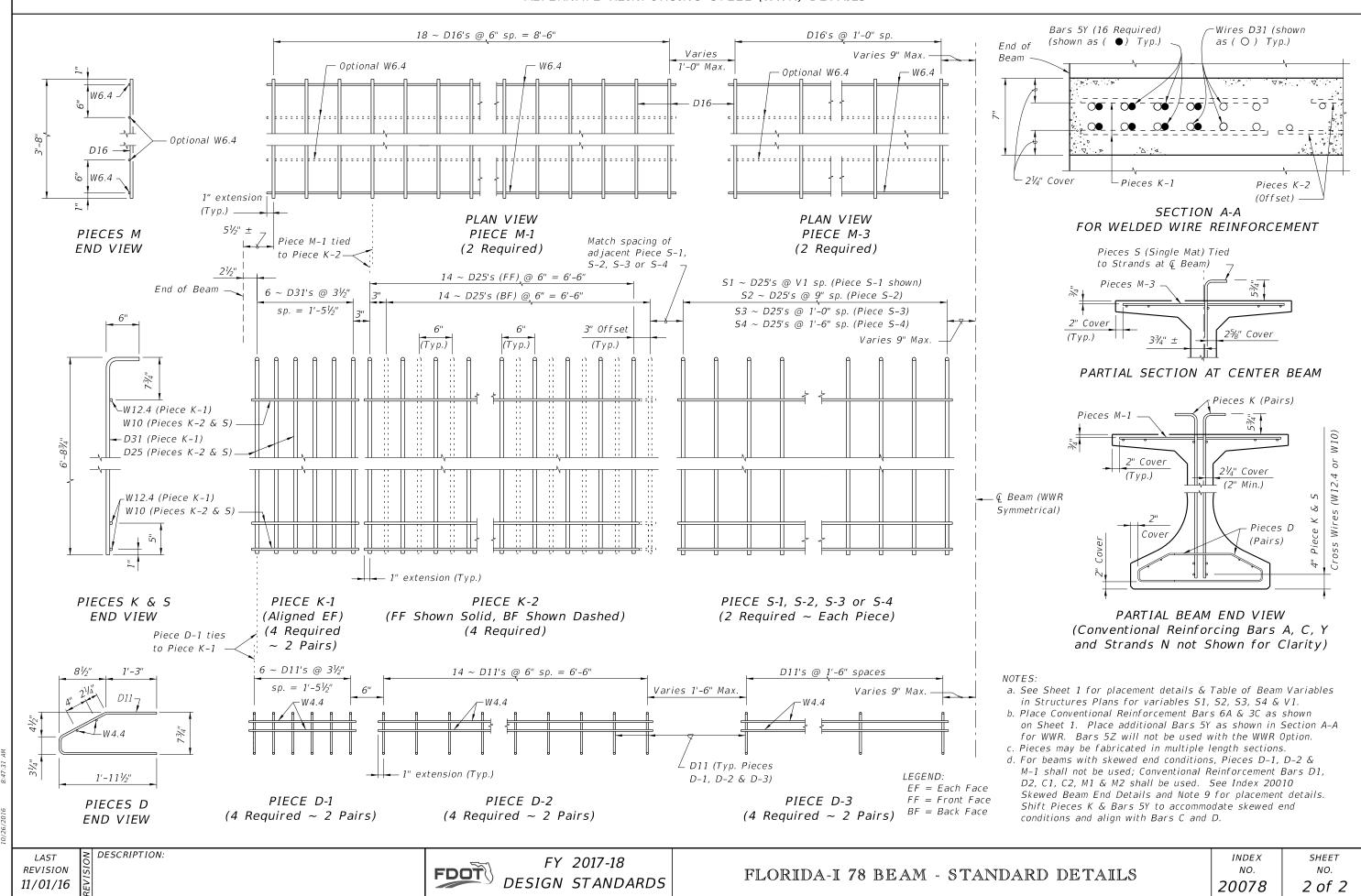


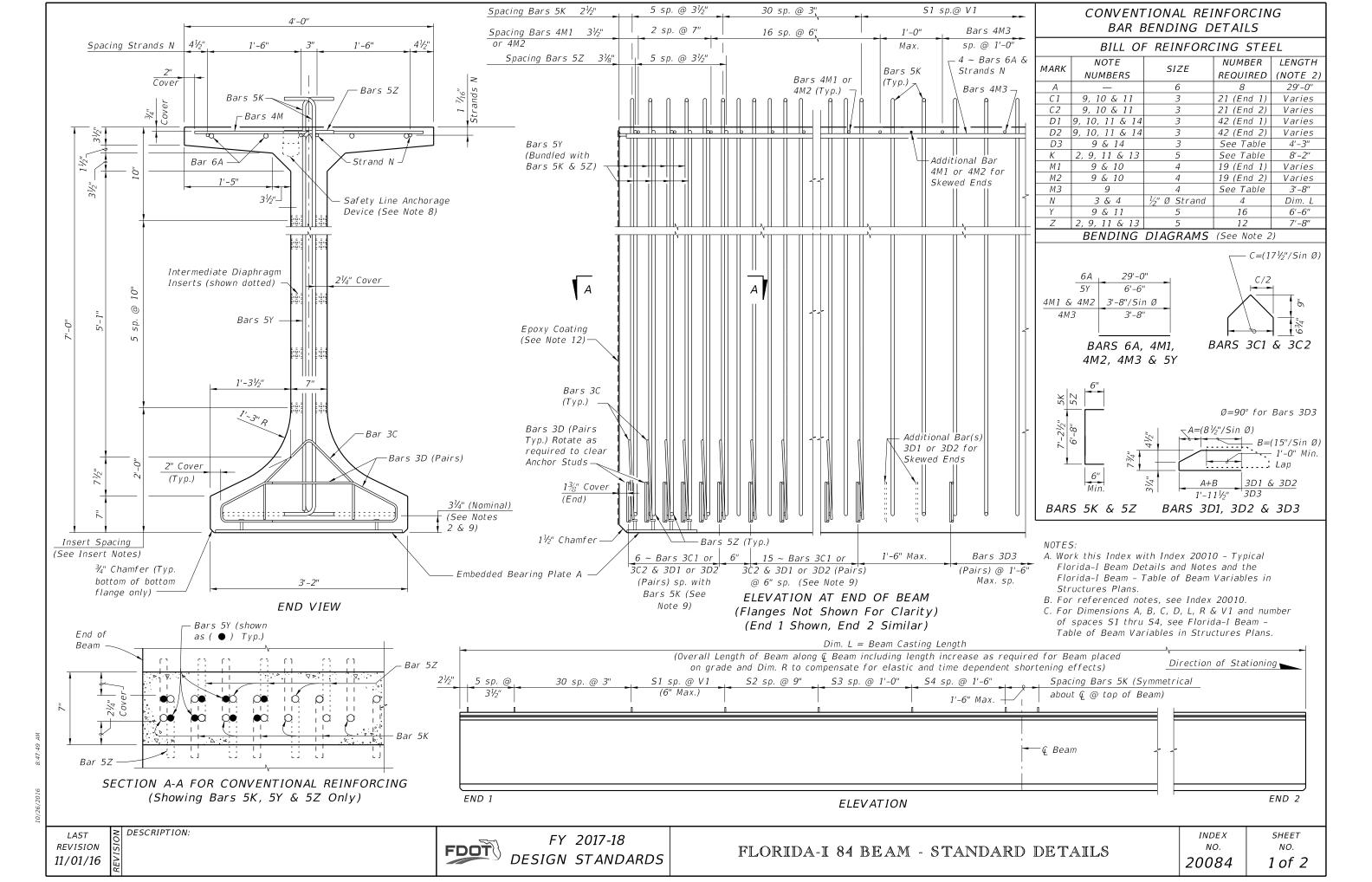


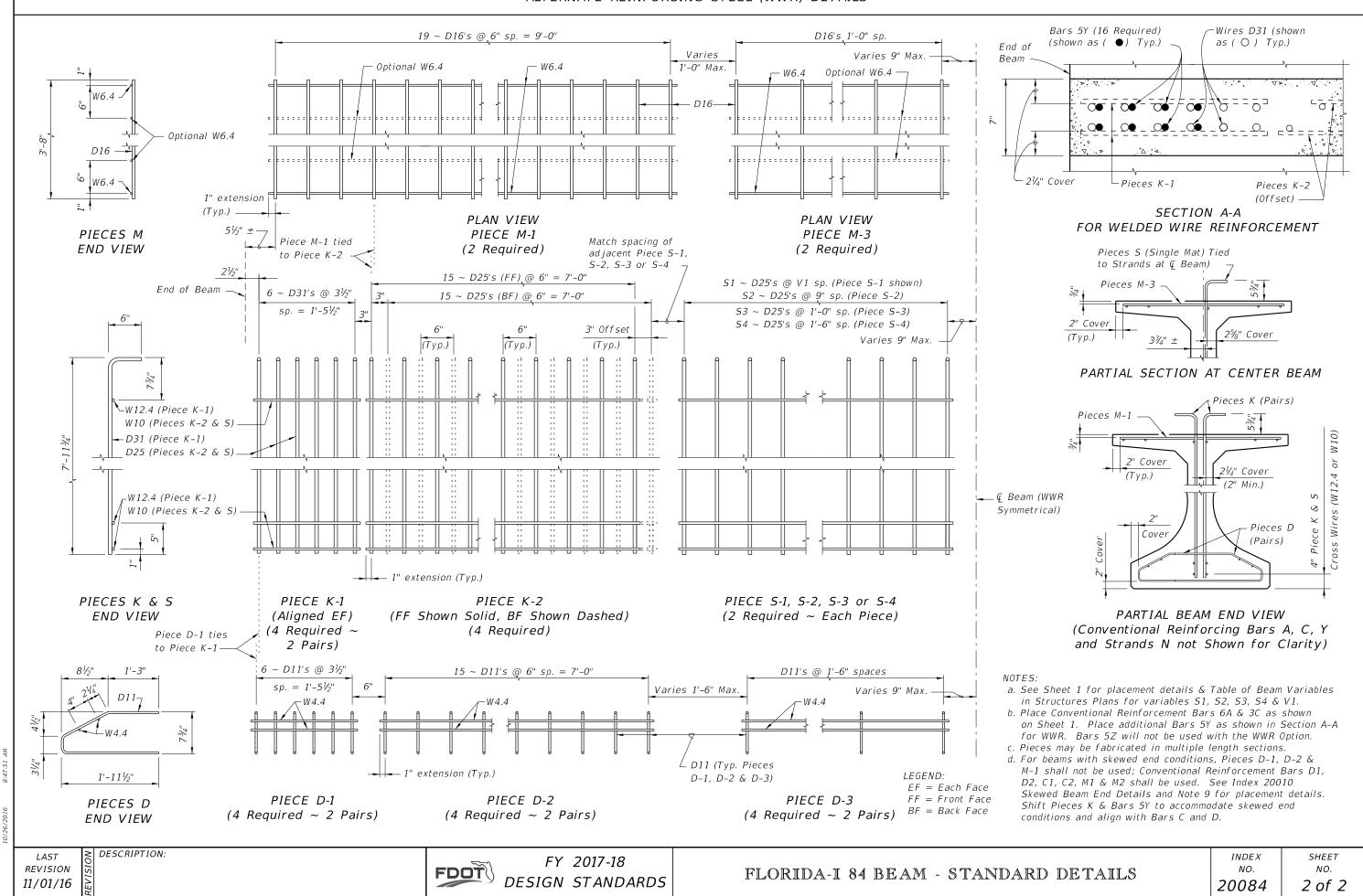


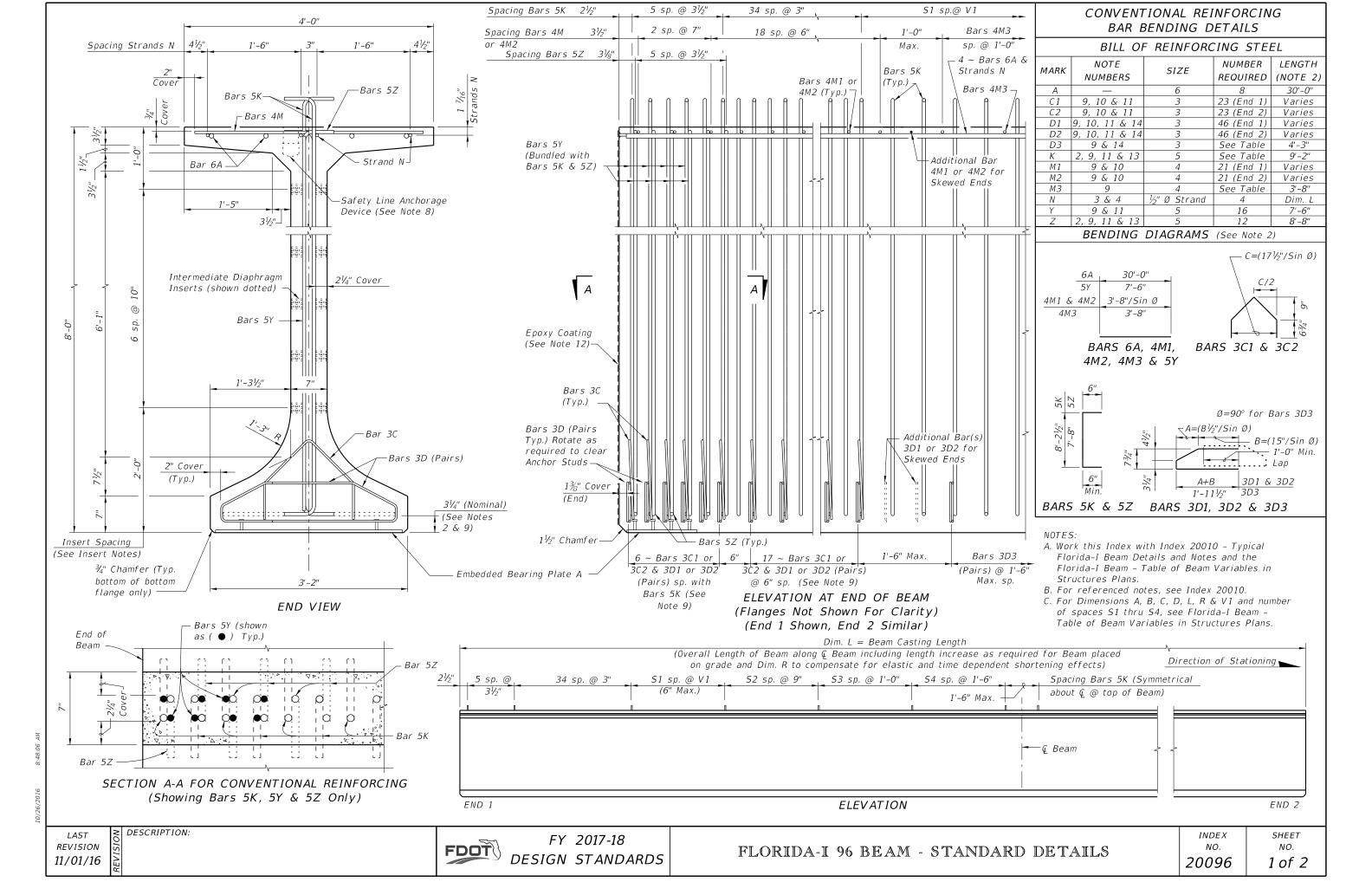


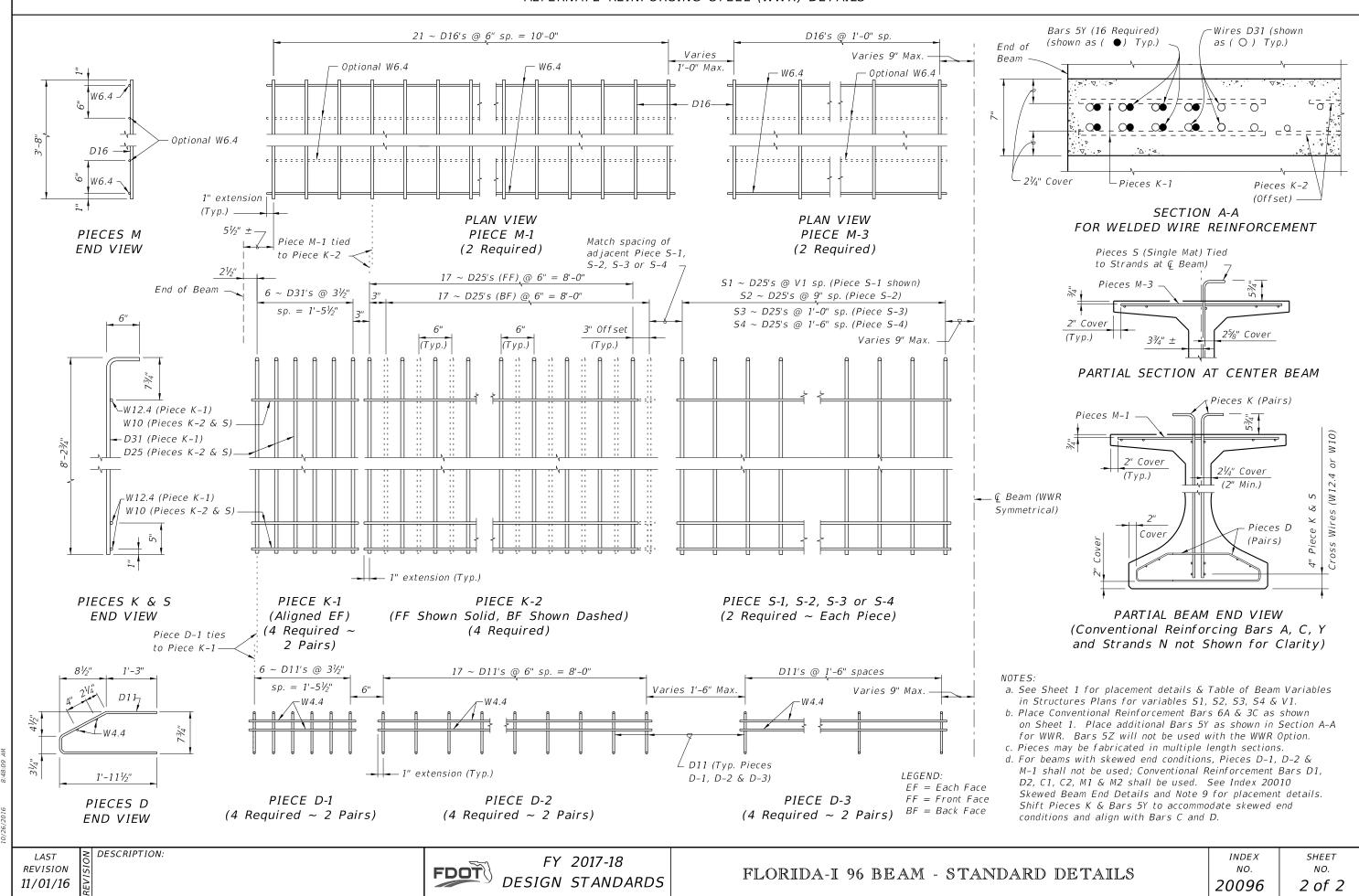


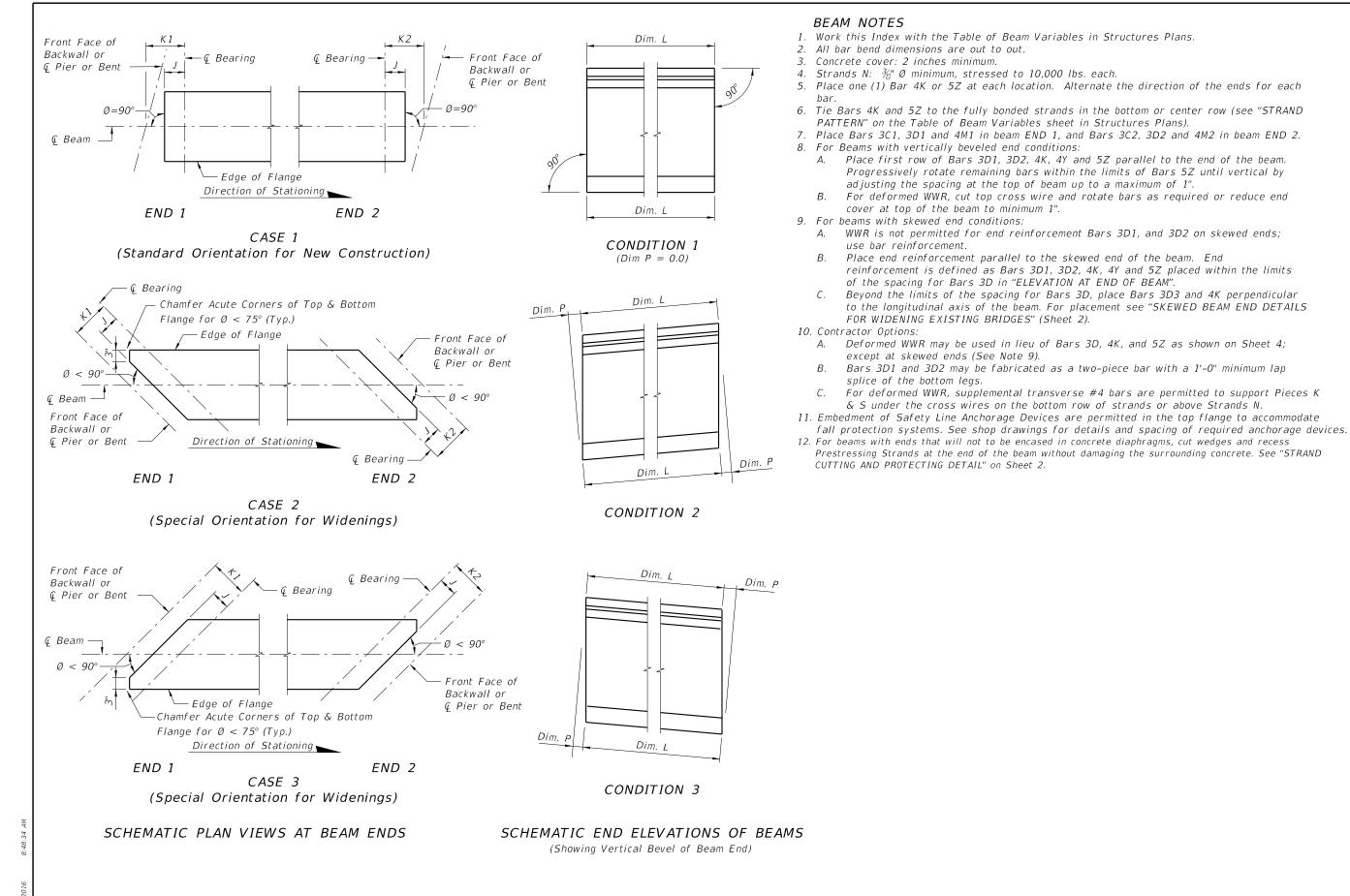












DETAILS AND NOTES

REVISION 11/01/16

DESCRIPTION:

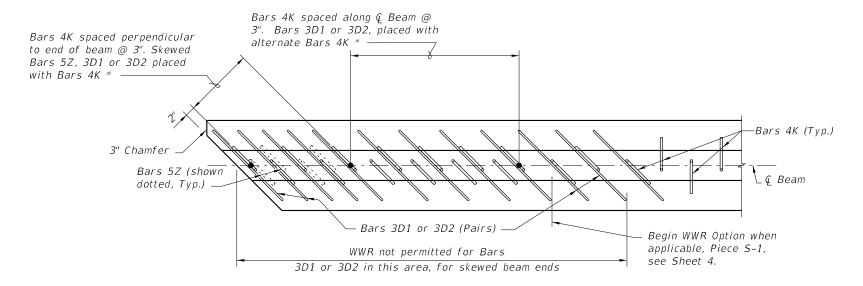
FDOT

FY 2017-18 DESIGN STANDARDS of the spacing for Bars 3D in "ELEVATION AT END OF BEAM".

splice of the bottom legs.

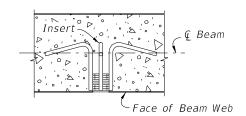
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

\* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for WWR.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE)
(End 1 Shown, End 2 Similar)
(Bars 4Y & Strands not shown for clarity)

= SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



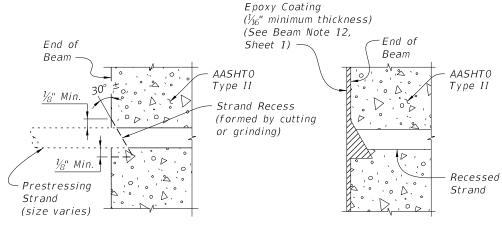
# PLAN SECTION THRU BEAM WEB AT INSERT FOR DIAPHRAGM REINFORCING

(When Intermediate Diaphragms are Required by Design)

#### **INSERT NOTES**

- 1. Provide 1" Ø, zinc-electroplated, ferrule wing nut or coil inserts, UNC threads, 1/0 minimum gage wire, not more than 4" in depth with a minimum ultimate tensile strength of 11,400 lbs. in 4,000 psi concrete.
- 2. If inserts are needed on both sides (faces) of beam webs, an assembly as long as the thickness of the beam web, consisting of two (2) ferrule or coil inserts attached by two (2) or more struts may be utilized. The connecting struts shall have a minimum ultimate tensile strength of 11,400 lbs.
- 3. Inserts for diaphragm reinforcing are required at each end of each intermediate diaphragm shown on the Beam Framing Plan and may be required at the end of the beams when end diaphragms are shown. See Superstructure and Beam Framing Plans for longitudinal location of inserts for each face of beam.

==== INSERT DETAIL ====



TYPICAL SECTION
SHOWING CUT STRAND RECESS LIMITS

TYPICAL SECTION
AFTER PROTECTING

=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

LAST CONTREVISION TO THE PROPERTY OF THE PROPE

DESCRIPTION:

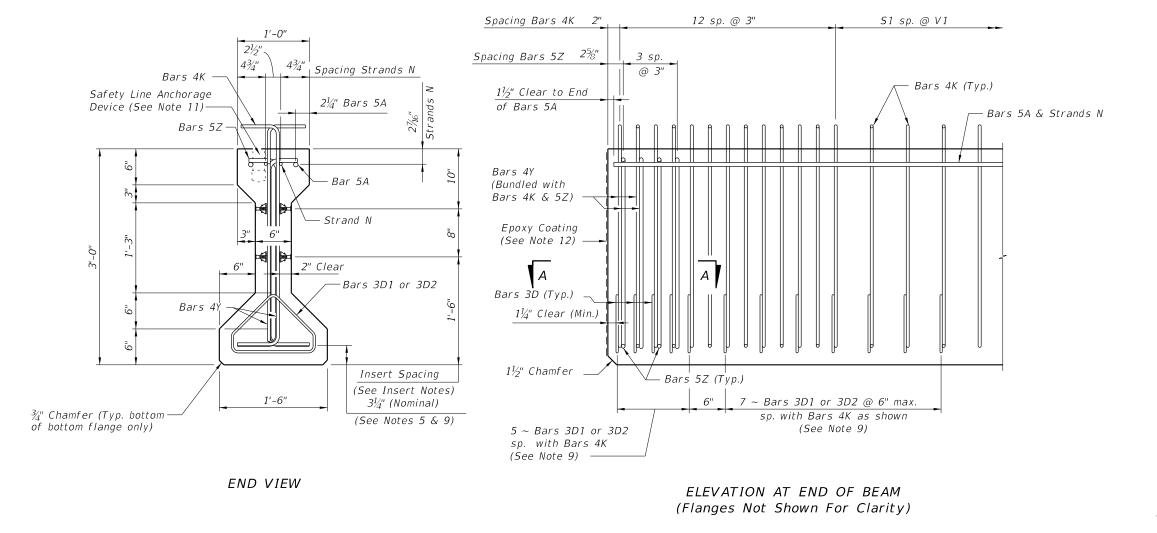
FDOT

FY 2017-18 DESIGN STANDARDS

AASHTO TYPE II BEAM

INDEX NO. **20120** 

SHEET NO. 2 of 4

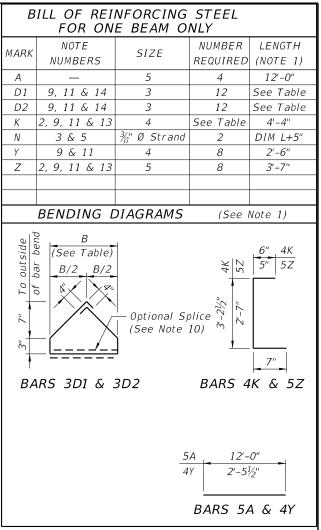


(Showing Bars 4K, 4Y & 5Z Only)

DESCRIPTION:

**REVISION** 

11/01/16



#### NOTES

AASHTO TYPE II BEAM

Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.

STANDARD DETAILS

SHEET

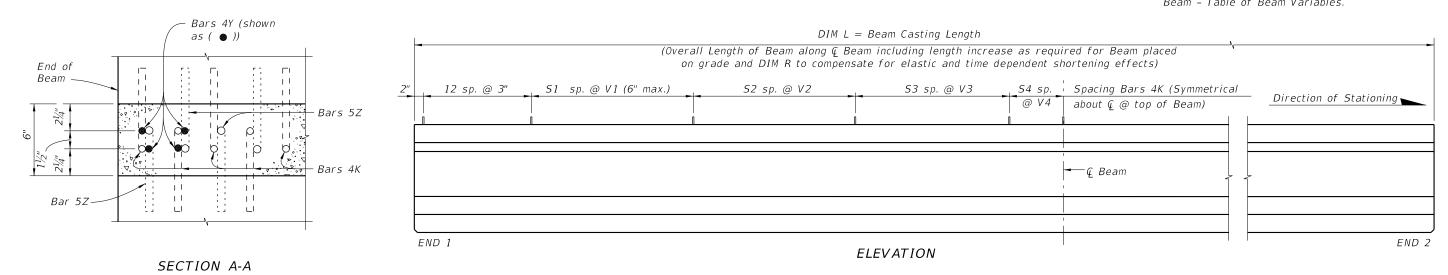
NO.

3 of 4

INDEX

NO.

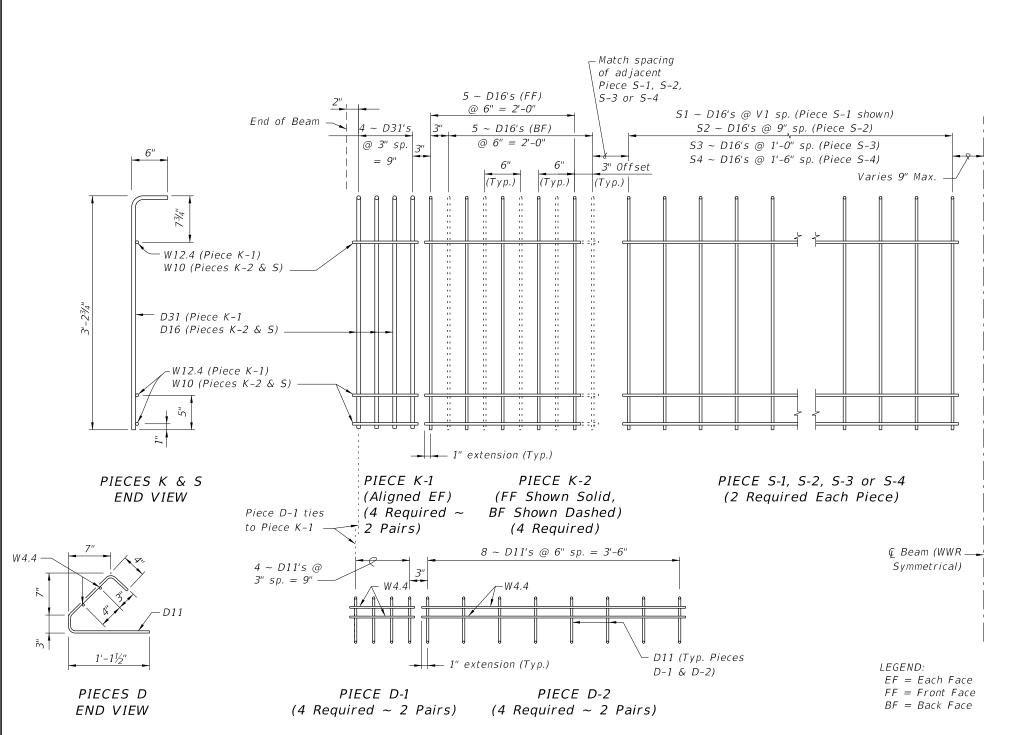
20120

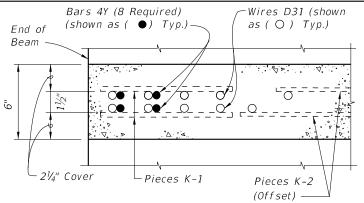


FY 2017-18

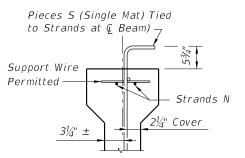
**DESIGN STANDARDS** 

FDOT

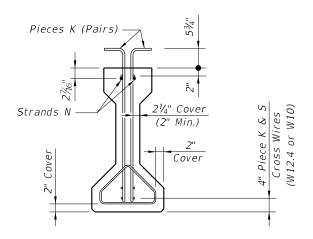




## SECTION A-A FOR WELDED WIRE REINFORCEMENT



#### PARTIAL SECTION AT CENTER BEAM



## PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

#### NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, 54 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

## STANDARD DETAILS

**REVISION** 11/01/16

DESCRIPTION:

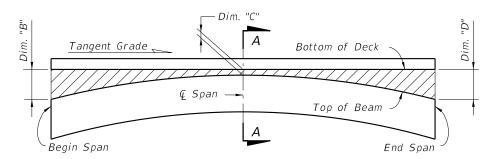
FDOT

FY 2017-18 **DESIGN STANDARDS** 

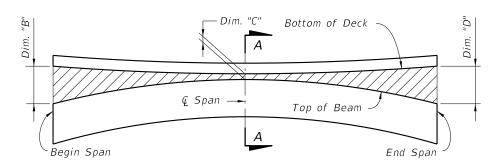
AASHTO TYPE II BEAM

INDEX NO. 20120

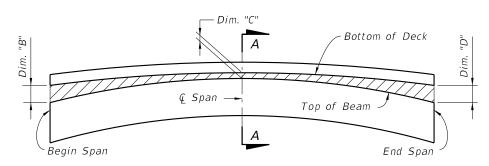
SHEET NO. 4 of 4



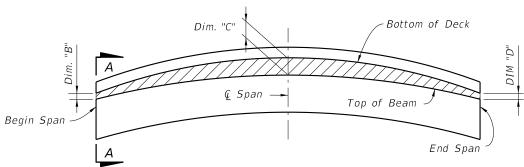
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG Q BEAM) (CASE 3)

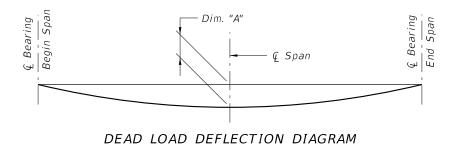


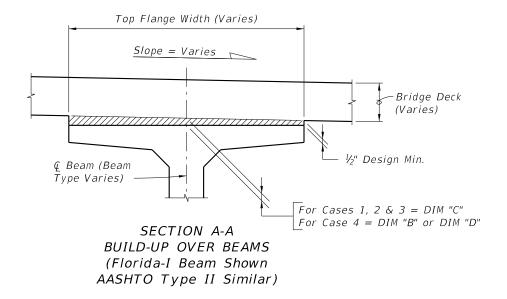
BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG Q BEAM) (CASE 4)

#### BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table\* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table\*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

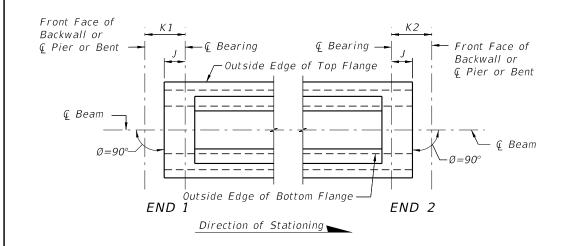
Dim. "A" includes the weight of the Stay-In-Place Formwork.



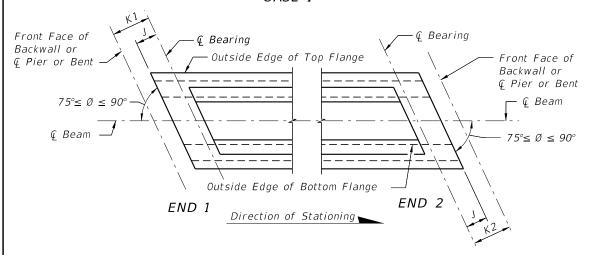


#### \* NOTE:

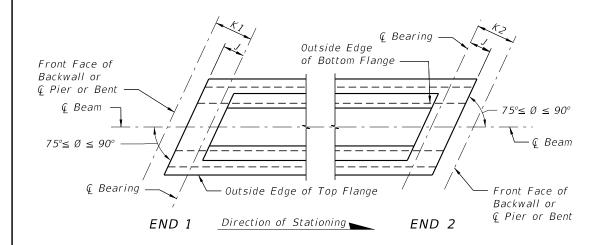
Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.



#### CASE 1



CASE 2



CASE 3

SCHEMATIC PLAN VIEWS AT BEAM ENDS =

LAST REVISION 11/01/16

DESCRIPTION:

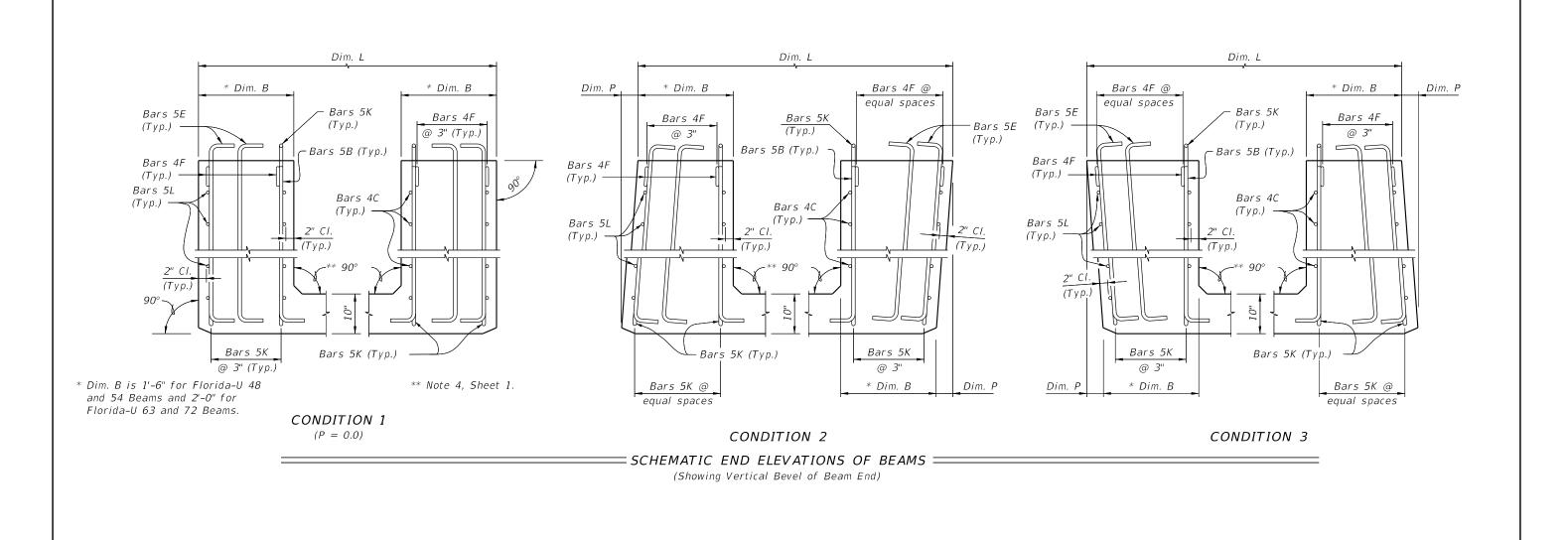


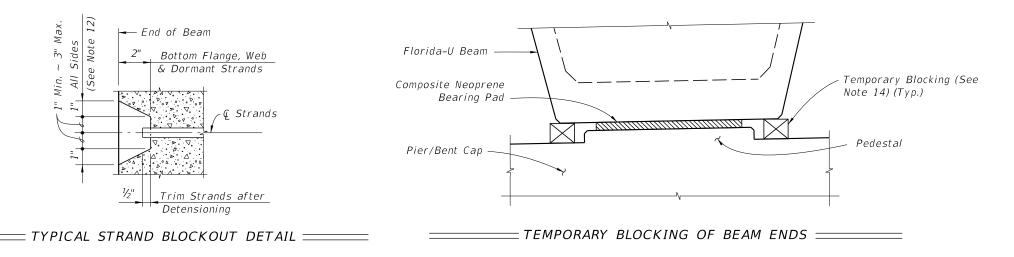
FY 2017-18

DESIGN STANDARDS

#### BEAM NOTES

- 1. Work this Index with the Florida-U Beam Standard Details (Index 20248, 20254, 20263 and 20272) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum. Maximum aggregate size is a No. 67.
- 4. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 5. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 6. Tie Bars 5K to the fully bonded strands in the bottom row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. For beams without skewed ends or vertically beveled end conditions (see Note 8) the Engineer may approve the use of deformed WWR in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M. The spacing and sizes of deformed WWR must match the reinforcing sizes shown on the Florida-U Beam Standard Details sheets.
- 8. For Beams with vertically beveled end conditions, where "Dim. P" exceeds 1", place Bars 5E, and the first Bars 4F and 5K parallel to the end of the beam. Fan the remaining Bars 4F and 5K within the limits of "Dim. B" (End Diaphragm) at equal spaces until vertical.
- 9. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any anchorage devices or other required embedded hardware.
- 10. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing the beam from casting bed.
- 11. Place drains pipes adjacent to each web at each beam end (four drains per beam).
  - A. Drain Pipe: 2" NPS Schedule 80 PVC.
  - B. Cover, wrap and secure wire screen around the end of the pipe prior to casting. Extend screen a minimum of 1" down the pipe sides.
  - C. Provide removable pipe plugs during casting. Remove plugs from the inside of pipes after casting.
- 12. Protection of Strands:
  - A. Provide a 2" deep recess around all strands (including dormant) or strand groups. Extend the recessed blockout to the web face and bottom of the flange for the bottom row of strands.
  - B. After detensioning, cut strands ½" from recessed surface and fill the blockout to protect strands with Type F-2 or Q Epoxy Compound in accordance with Specification Section 926.
- 13. Use Stay-In-Place metal deck forms inside the beams.
- 14. Prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations during deck placement. Leave temporary blocking and bracing in place for a minimum of four days after the deck is placed.
- 15. Based on the deck forming system and deck placement sequence, evaluate and provide any required temporary bracing between the U Beams.





REVISION 11/01/16

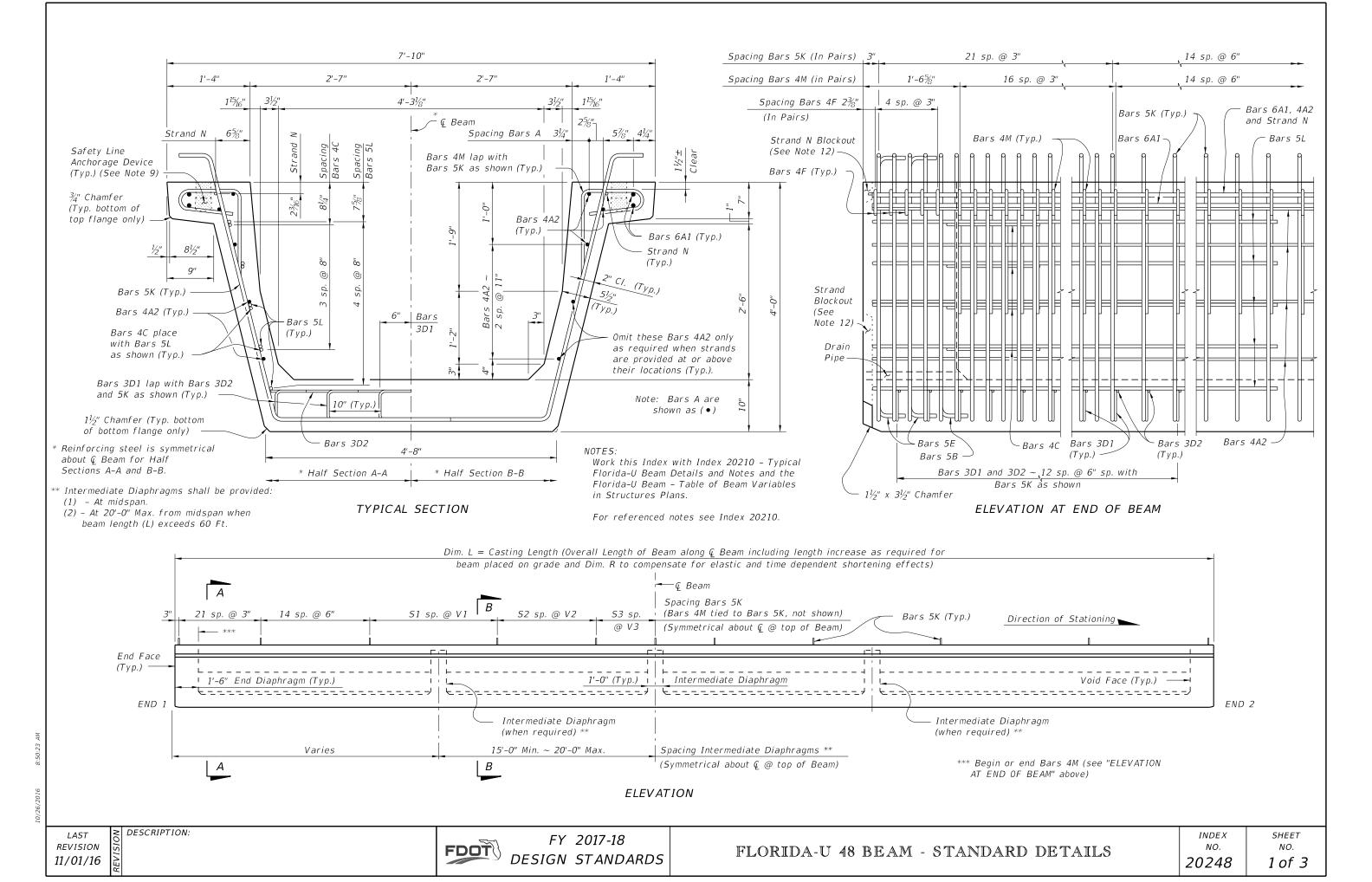
DESCRIPTION:

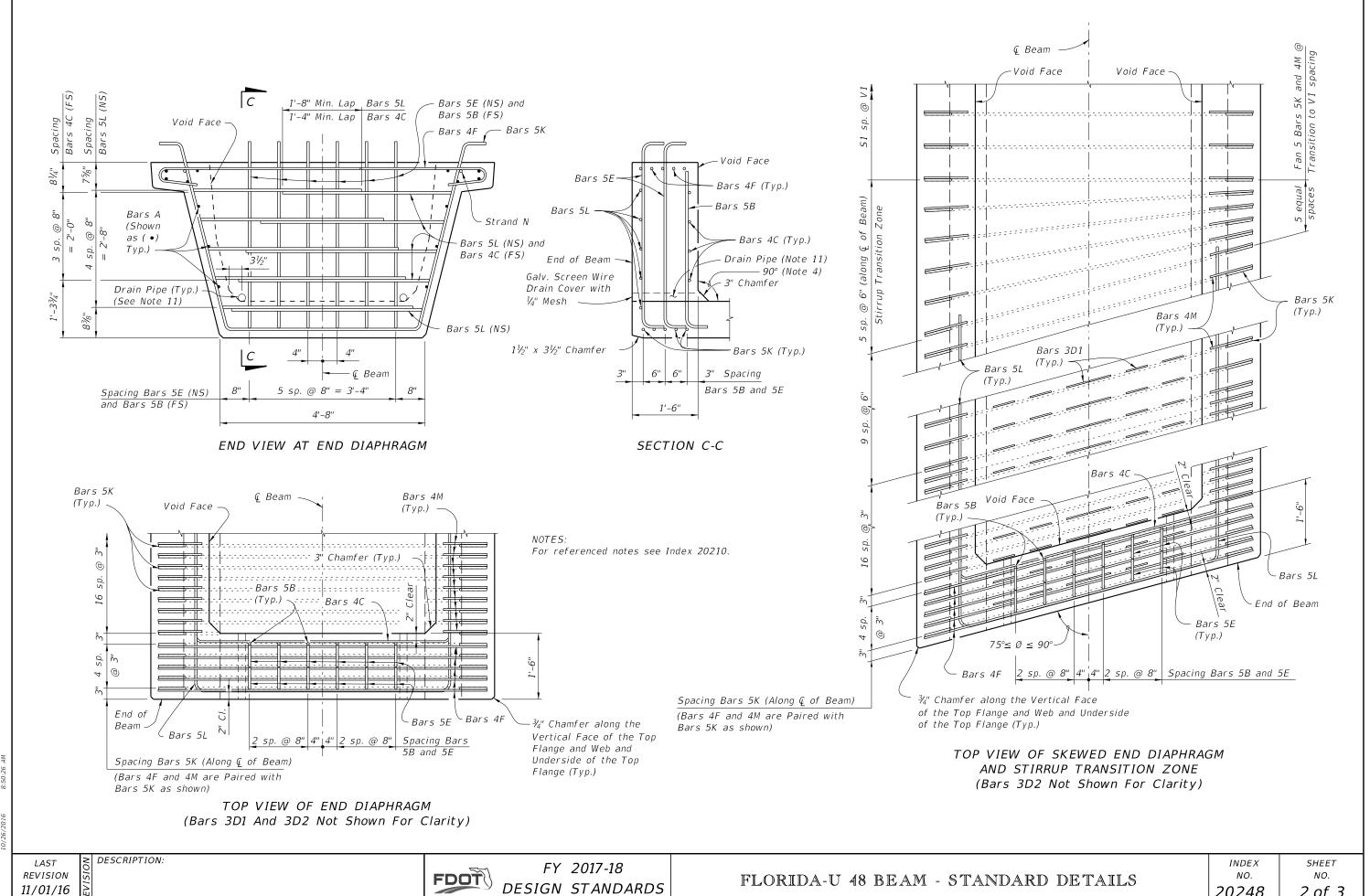
FDOT

FY 2017-18 DESIGN STANDARDS

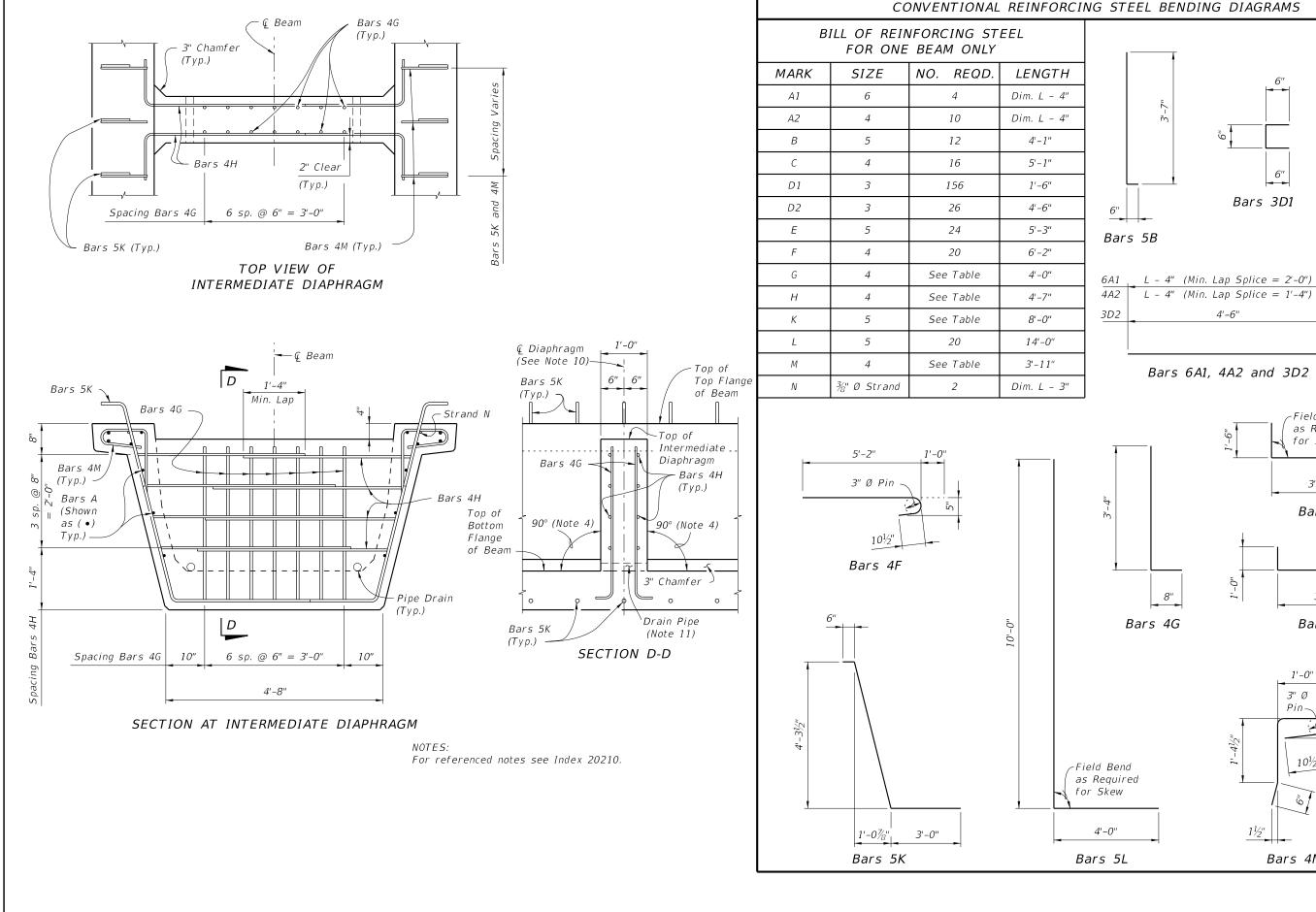
INDEX NO. 20210

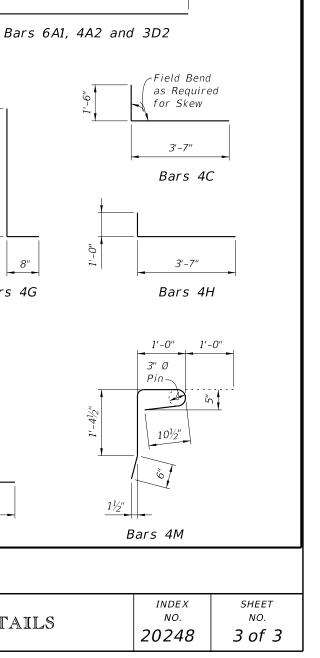
SHEET NO. 2 of 2





DESIGN STANDARDS



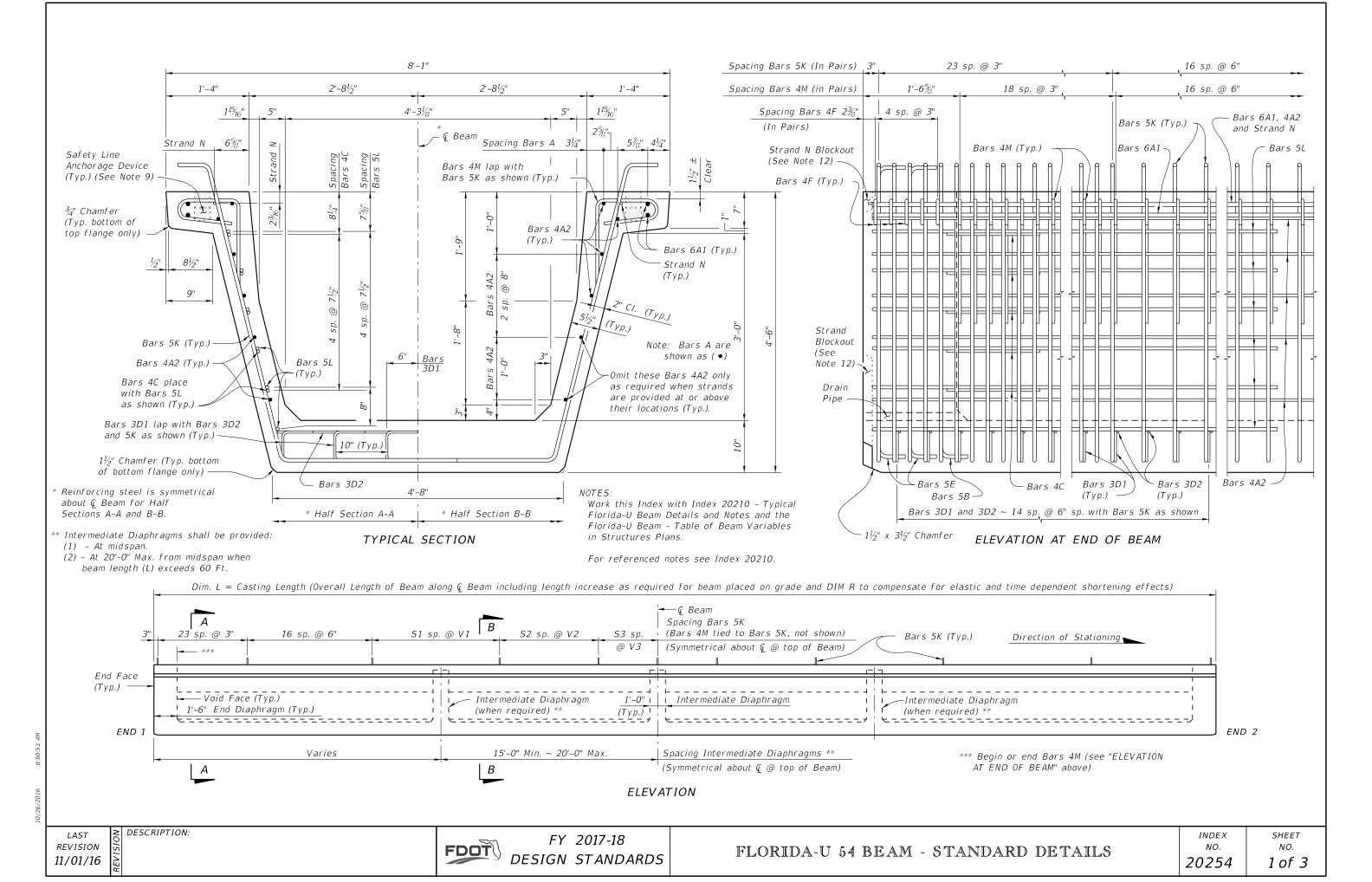


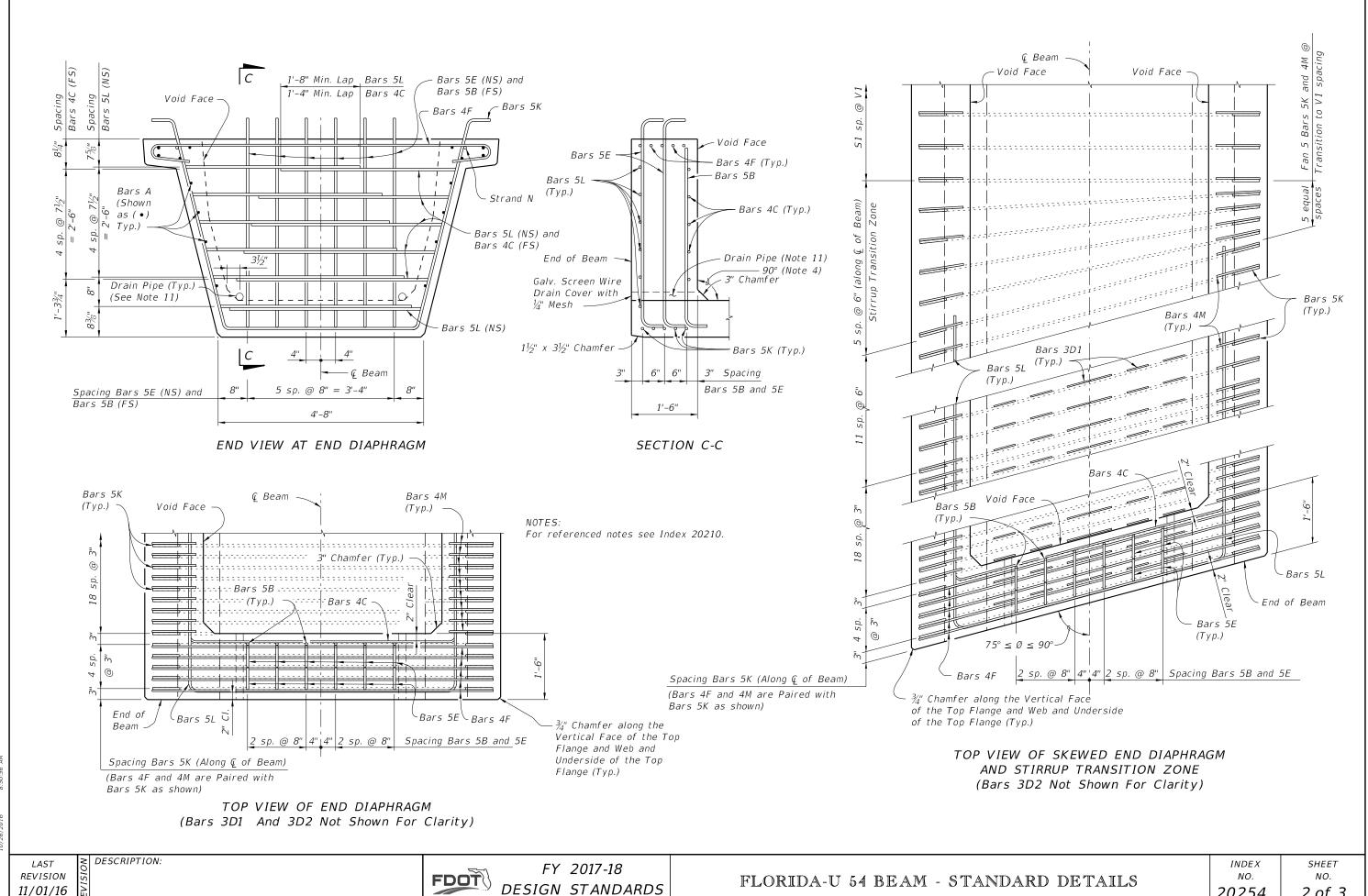
Bars 3D1

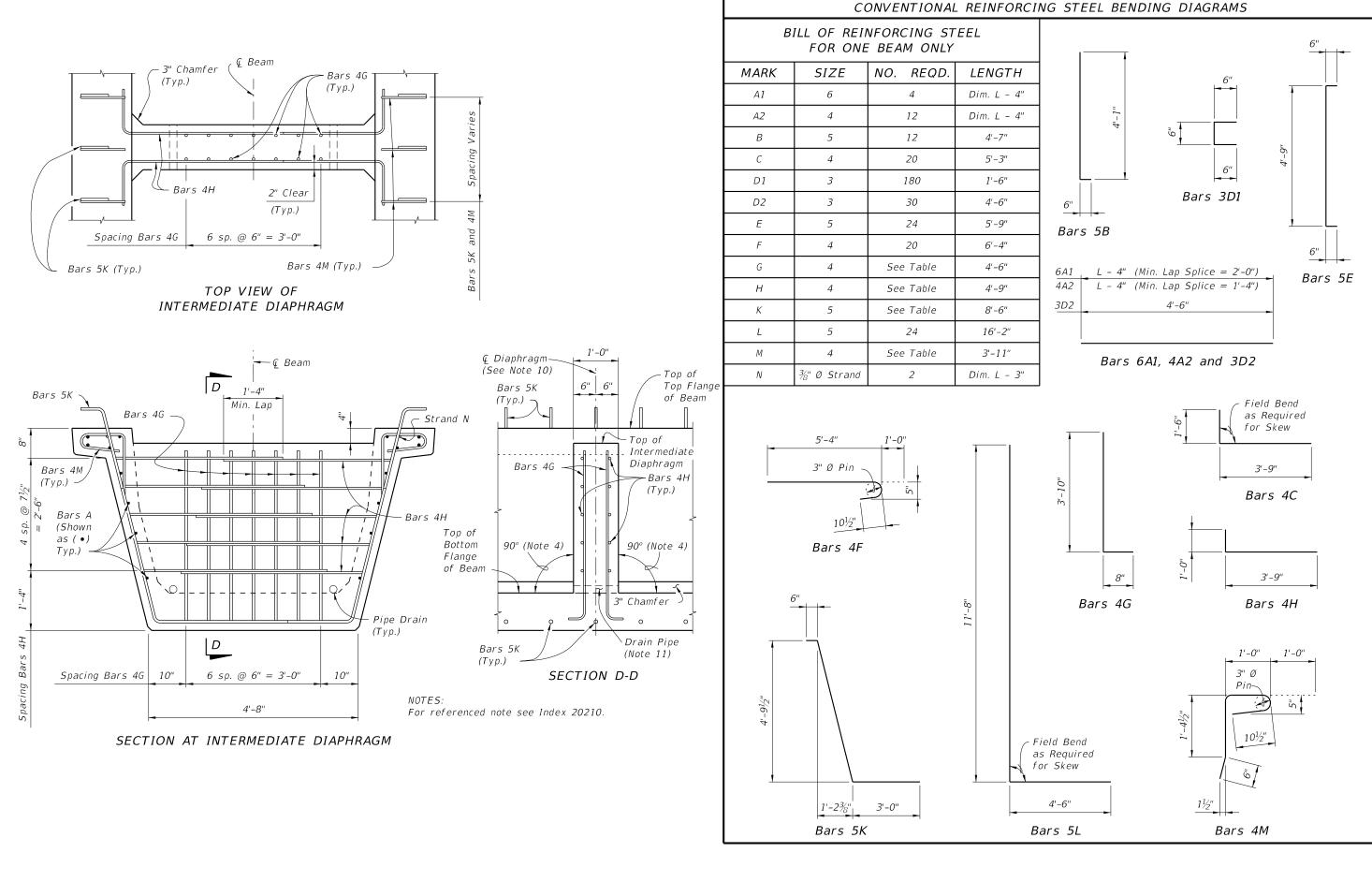
4'-6"

Bars 5E

11/01/16

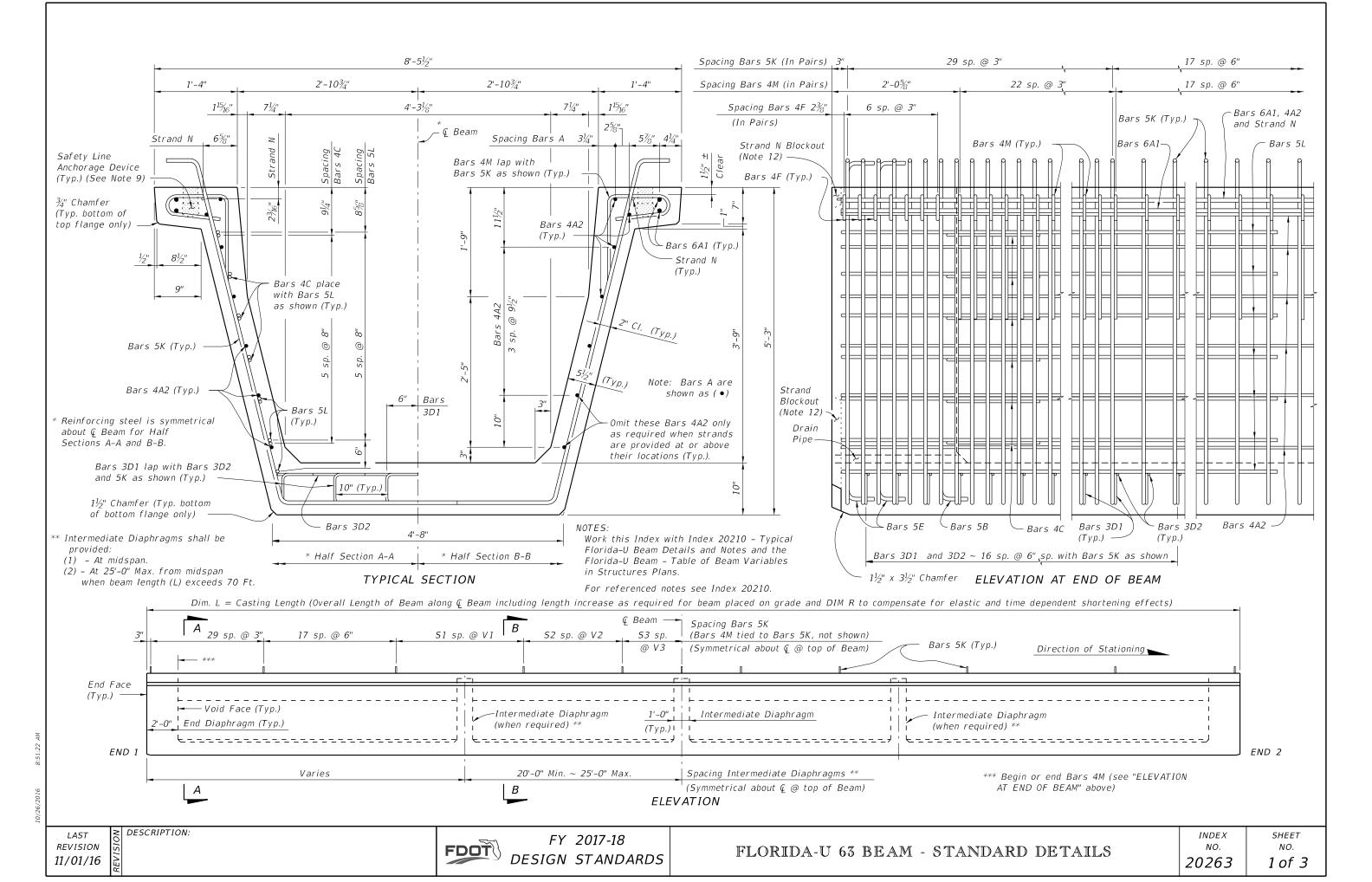


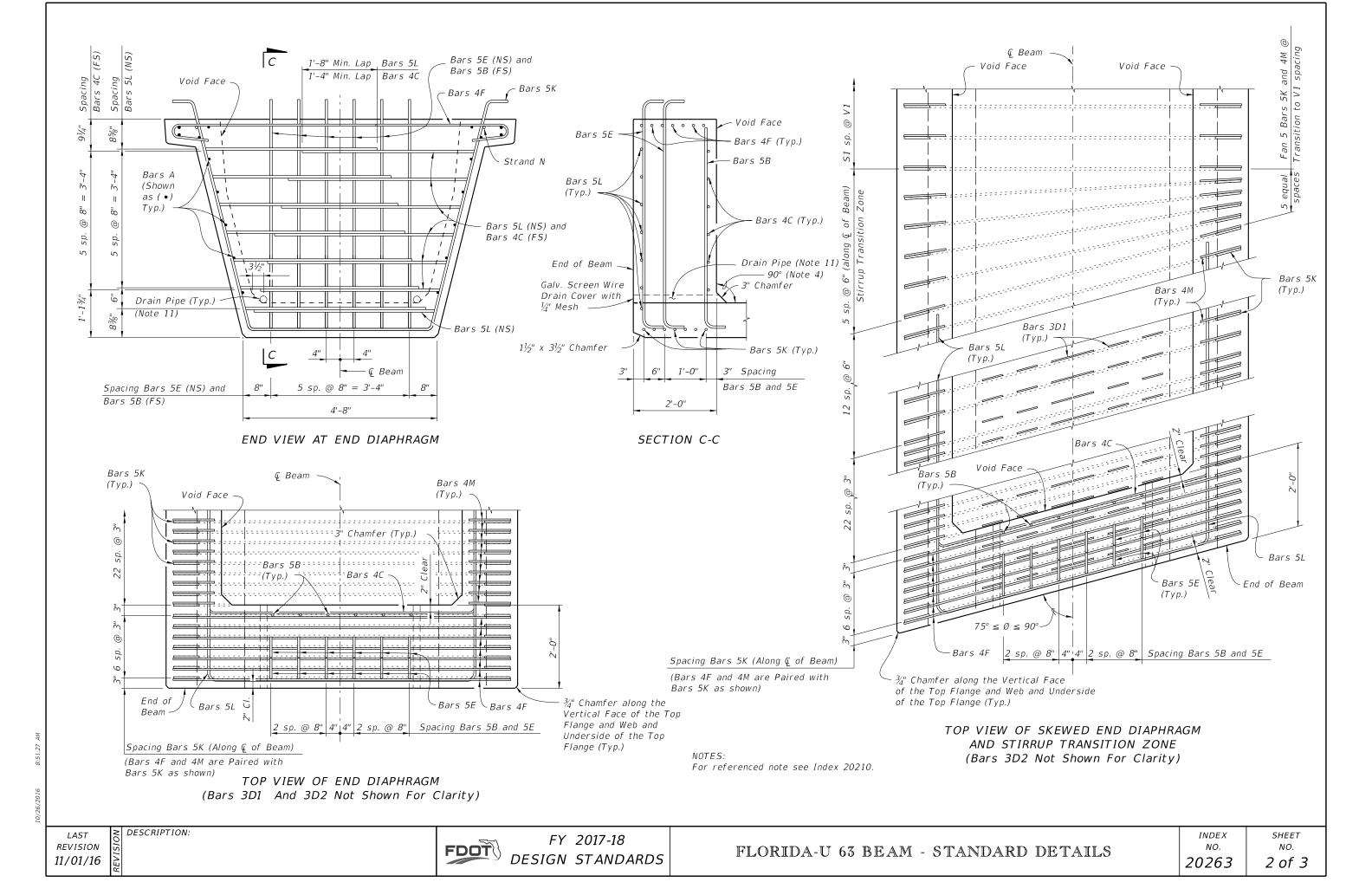


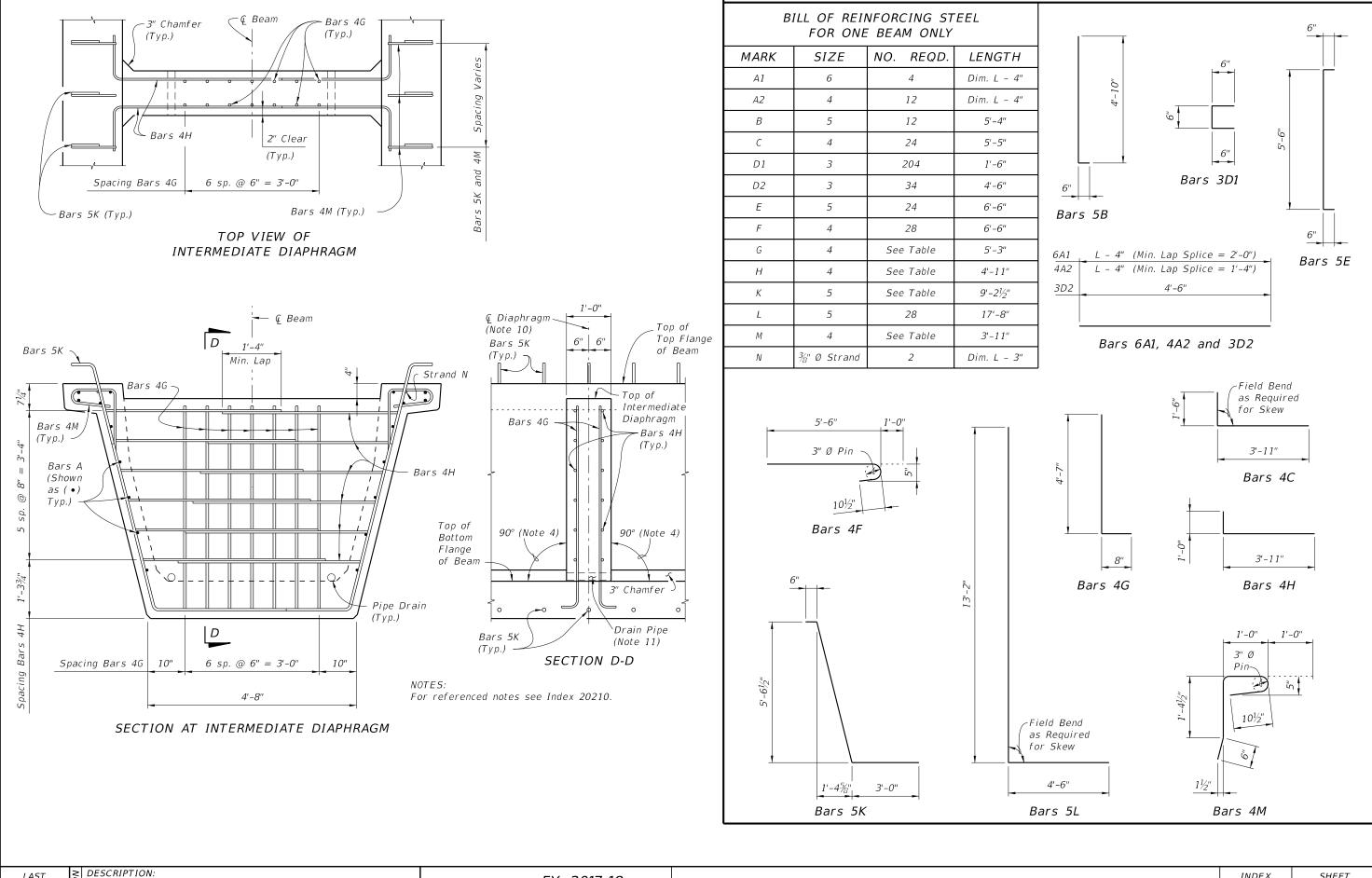


**REVISION** 11/01/16

DESCRIPTION:





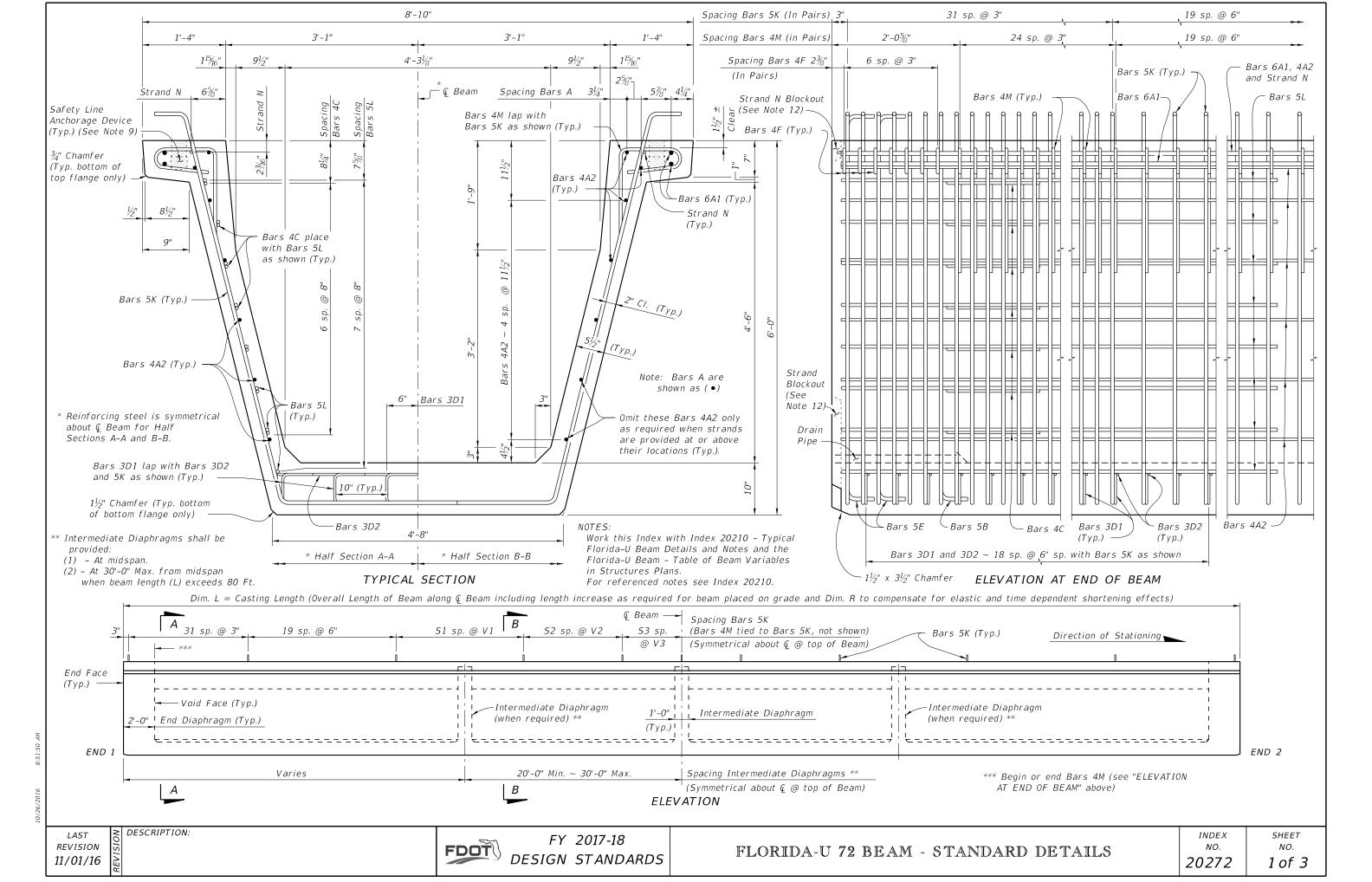


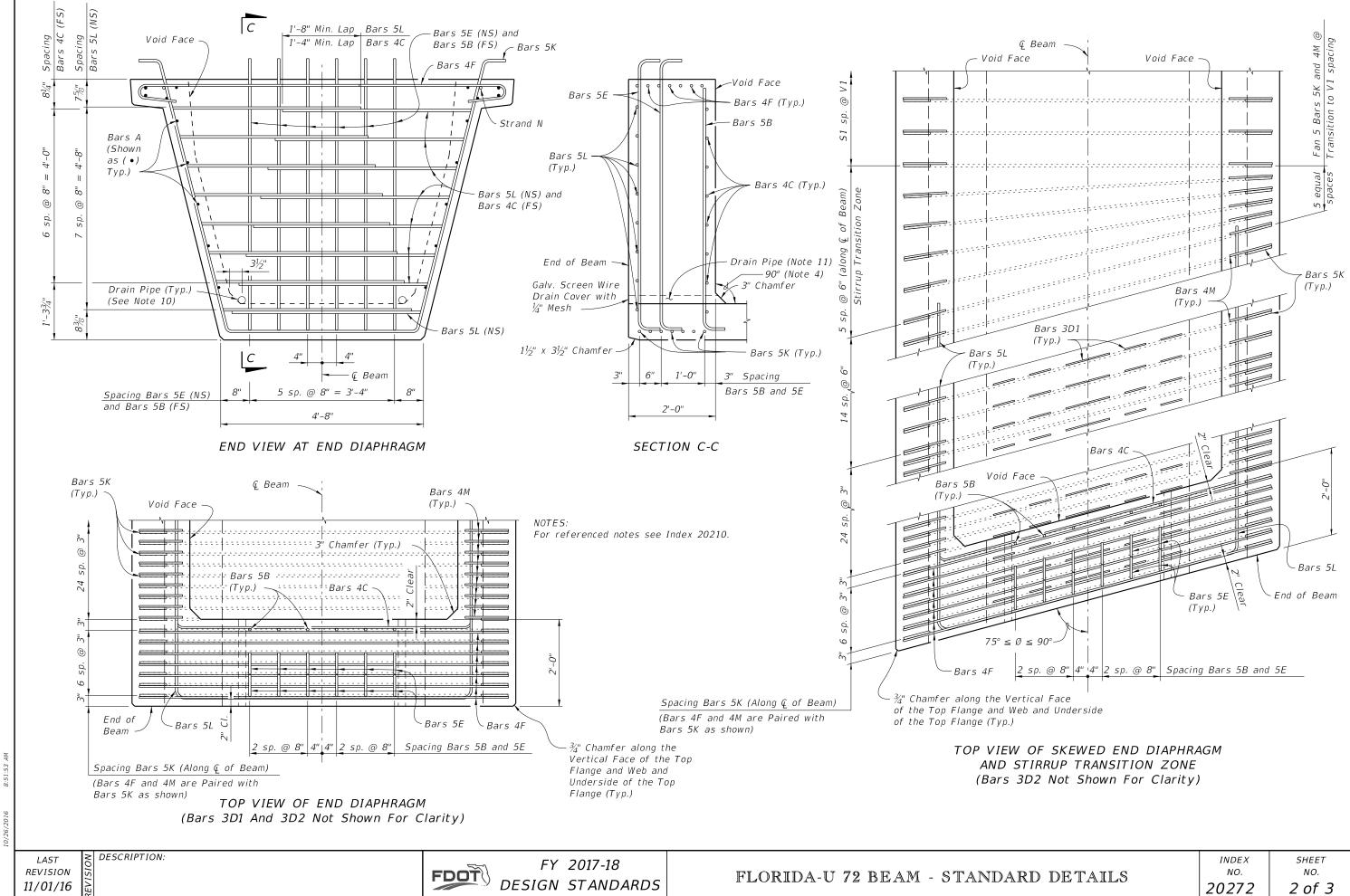
**REVISION** 

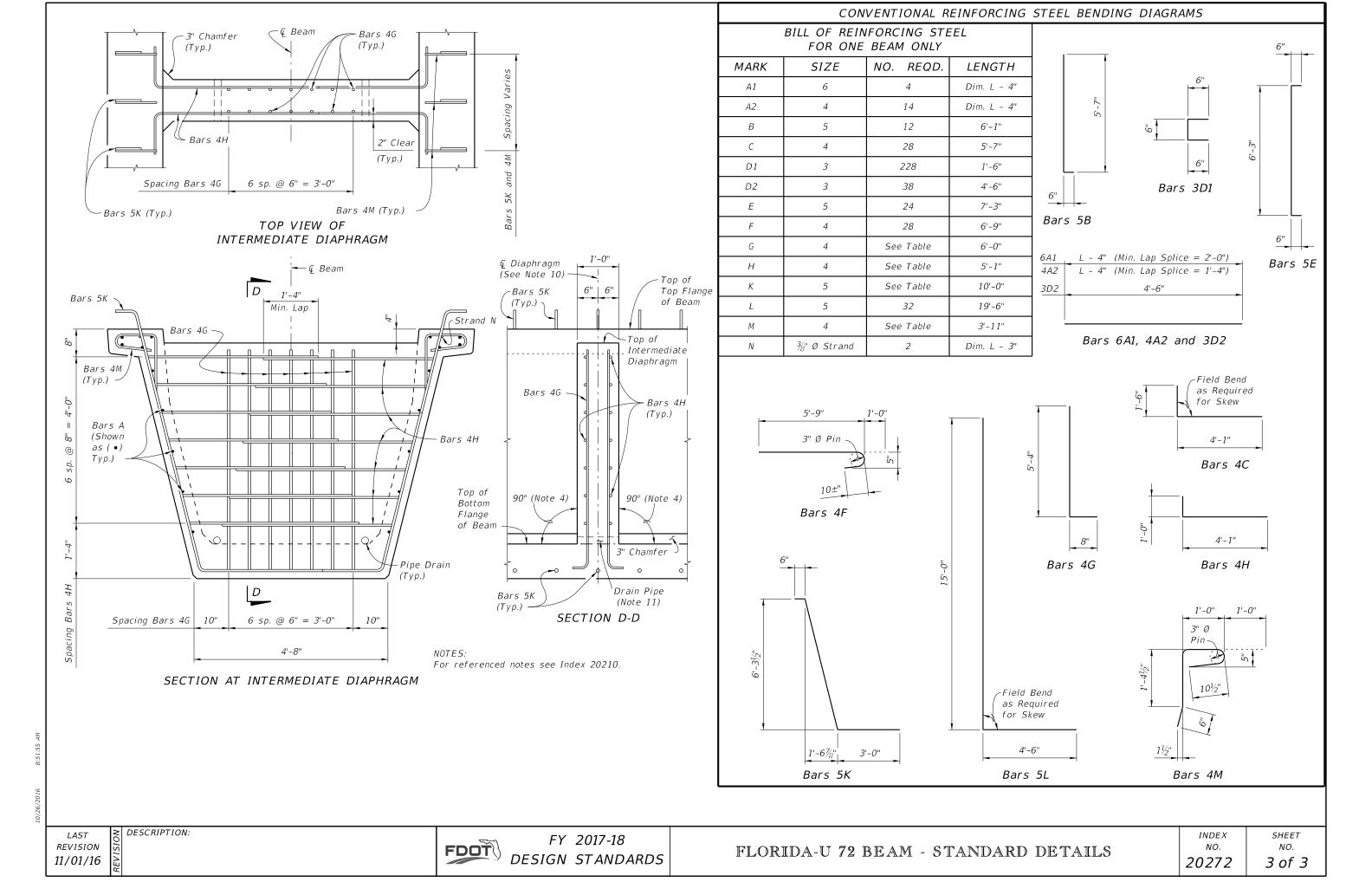
11/01/16

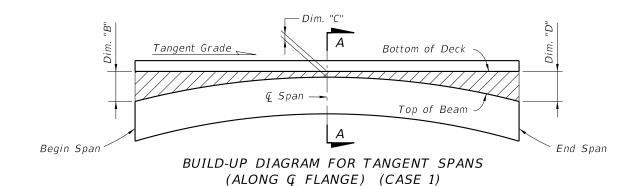
FDOT

FY 2017-18 DESIGN STANDARDS CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS



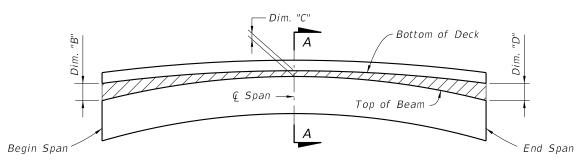




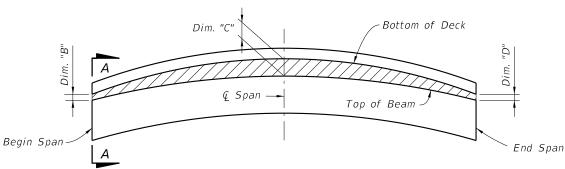


## Bottom of Deck Α Top of Beam End Span Begin Span

BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q FLANGE) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG G FLANGE) (CASE 3)

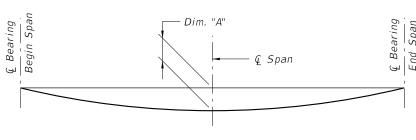


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG G FLANGE) (CASE 4)

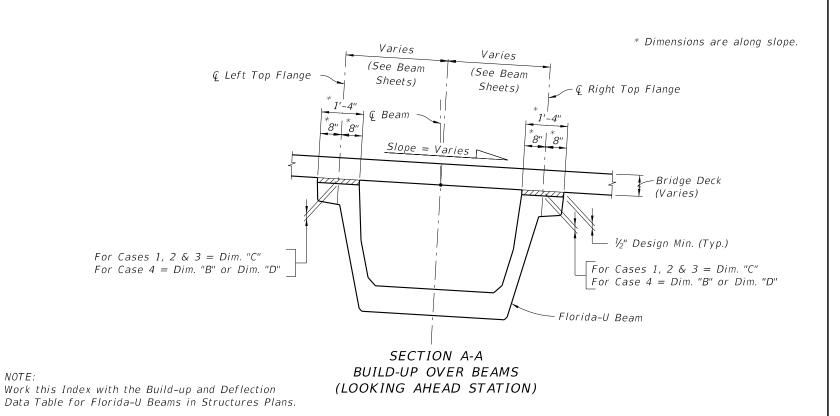
#### BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table\* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table\*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM (ALONG G BEAM)



DESCRIPTION: **REVISION** 07/01/15

FDOT

FY 2017-18 DESIGN STANDARDS