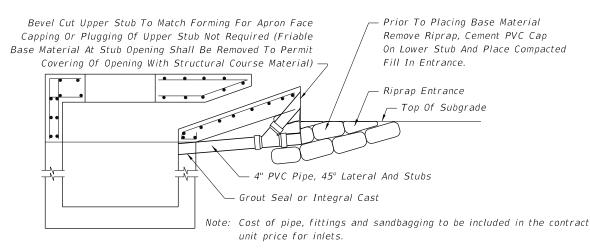


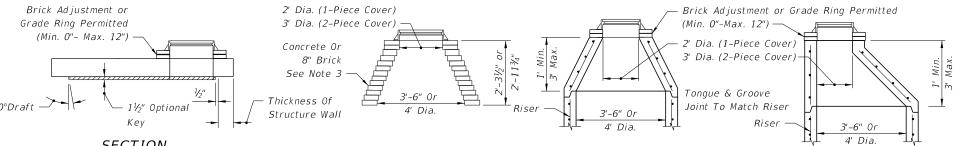
NOTE: When Alternate "G" grate is specified, the chain, bolt, nuts, washer and cold shuts shall be galvanized in accordance with Section 425 of the Standard Specifications.

Cost of eyebolt and chain to be included in the contract unit price for inlets.

EYEBOLT AND CHAIN REQUIREMENTS										
Index Number	Inlet Type	Eye- Bolts	Length Of Chain	Handling & Remarks						
Namber	(MB) 1	1	4'-0"	Slide & Spin						
217	(MB) 2	1	4'-0"	Slide & Spin						
	(MB) 3	2	2 @ 4'-0"	Slide & Spin						
	(MB) 4	2	2 @ 4'-0"	Slide & Spin						
	(MB) 5	2	2 @ 4'-0"	Slide & Spin						
218	(BW)	1	3'-8"	Slide Or Slide & Spin						
219	(BW, RGD)	1	4'-0"	Slide & Spin						
220	5	1	4'-0"	Slide & Spin						
221	V	1	4'-0"	Slide & Spin						
230	А	1	3'-0"	Slide						
231	В	1	5'-0"	Slide & Spin						
	С	1	2'-6"	Slide & Spin						
	D	1	2'-6"	Slide & Spin						
232	Ε	2	2 @ 2'-6"	Slide & Spin						
	Н	2	2 @ 2'-6"	Flip Ctr. Grate and Slide & Spin Single Free Grate						
			1 or 2 @ 1'-6"	Ctr. Grate(s) Chained To One End Grate						
233	F	1	3'-6"	Flip Or Slide & Spin						
	G	1	6'-0"	Slide						
			2'-0"	Lifting Loop						
234	J	1	4'-0"	Slide & Spin						

EYEBOLT AND CHAIN FOR LOCKING GRATES TO INLETS





SECTION

Note: See Slab Designs Index No. 200.

BRICK OR CONCRETE PRECAST CONCENTRIC CONE PRECAST ECCENTRIC CONE TYPE 8

MANHOLE TOPS

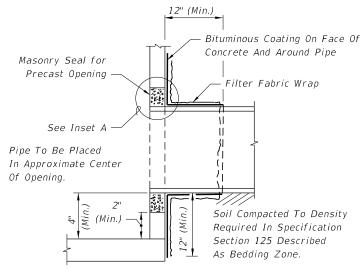
NOTES (TOPS)

TYPE 7

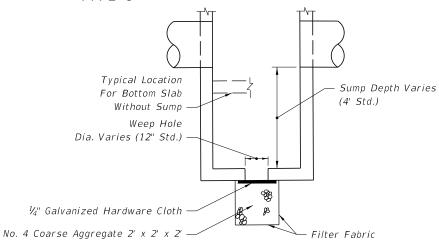
- 1. Manhole top Type 7 slabs shall be of Class II concrete. Concrete as specified in ASTM C478 may be used for precast units; see General Note No. 3.
- 2. Manhole top Type 7 slabs may be of cast-in-place or precast construction. The optional key is for precast tops and in lieu of dowels. Frame and slab openings are to be omitted when top is used over a junction box.
- 3. Manhole top Type 8 may be of cast-in-place or precast concrete construction or brick construction. For concrete construction, the concrete and steel reinforcement shall be the same as the supporting wall unit. An eccentric cone may be used.
- 4. Manhole tops shall be secured to structures by optional construction joints as shown on Sheet 3.
- 5. Frames can be adjusted a maximum 12" height with brick or precast ASTM C478 grade rings.
- 6. Substitution of manhole top Type 8 for manhole top Type 7 is allowed provided that minimum dimensions shown above are not reduced.
- 7. Substitution of Manhole top Type 7 for Type 8 is allowed if the minimum thickness (h) above pipe opening cannot be maintained with manhole top Type 8.

DESIGN NOTES

1. Manhole top Type 8 should be specified in the plans when depths shown above can be maintained.

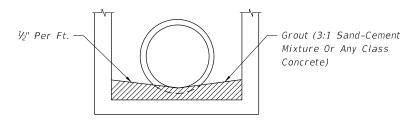


FILTER FABRIC WRAP ON GROUTED PIPE TO STRUCTURE JOINT

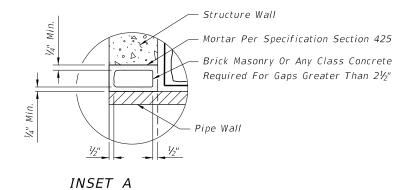


NOTE: Sump bottom appropriate for all manhole and inlet types. Sumps are to be constructed in inlet and manholes connected to French Drains unless excluded in the plans. At other locations, sump is to be constructed only where called for in the plans. Weep holes to be constructed in sump bottom only where called for in the plans. Cost of sump bottom and weep hole to be included in the contract unit price for inlet or manhole.

SUMP BOTTOM



FOR ALL STRUCTURES UNLESS EXCLUDED BY SPECIAL DETAIL ALL PIPE TYPES DRAINAGE STRUCTURE INVERT



TEMPORARY DRAINS FOR SUBGRADE AND BASE

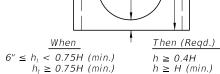
REVISION 07/01/12

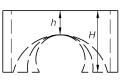
DESCRIPTION:

FDOT

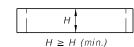
FY 2017-18 DESIGN STANDARDS

SHEET NO. 2 of 5



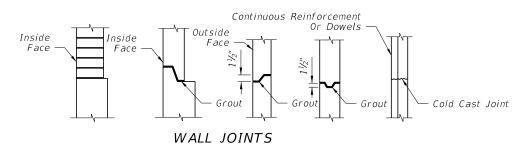


Segments may be inverted. Opening for pipe shall be the pipe OD plus 6" (\pm 2" tolerance). If h can not be attained, then a top or bottom slab must be attached to the segment as shown below.

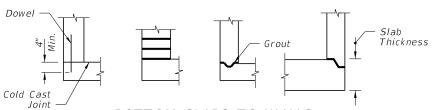


Minimum Value For H								
H (min.)	Box Or Riser Diameter							
1'-0"	3'-6" & 4'-0"							
1'-6"	5'-0" & 6'-0"							
2'-0"	>6'-0"							

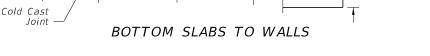
SEPARATE RISER SEGMENTS WITH CONSTRUCTION JOINTS OTHER THAN DOWEL OPTION



Fill Keyway With Grout (When Present)



TOP SLABS TO WALLS

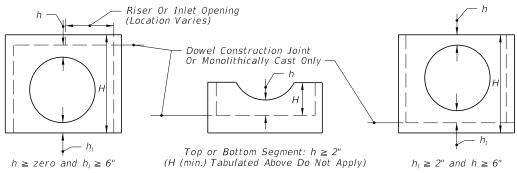


- 1. One or more types of joints may be used in a single structure, except brick wall structure. Brick wall construction is permitted on circular units only.
- 2. All grouted joints are to have a maximum thickness of 1".
- 3. Keyways are to be a minimum of $1\frac{1}{2}$ " deep.

DESCRIPTION:

- 4. Joint dowels are to be #4 bars, 12" long with a minimum of 6 bars per joint approximately evenly spaced for circular structures or at maximum 12" spacing for rectangular structures. Bars may be either Adhesive Bonded Dowels in accordance with Specification Section 416, or placed approximately 6" into fresh concrete leaving the remainder to extend into the secondary cast. Welded wire reinforcement may be substituted for the dowel bar in accordance with the equivalent steel area table on
- 5. Minimum cover on dowel reinforcing bars is 2" to outside face of structure.
- 6. Joints between wall segments and between wall segments and top or bottom slabs may be sealed either by preformed plastic gasket material using the procedures given in Section 430 of the Specifications or by non-shrink grout, in accordance with Section 934 of the Specifications.
- 7. Insert products approved by the Engineer may be used in lieu of dowel embedment.

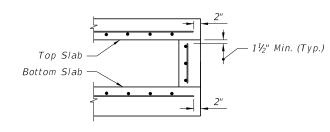
OPTIONAL CONSTRUCTION JOINTS



SEGMENTS FOR SLAB TO WALL DOWEL CONSTRUCTION JOINTS OR MONOLITHICALLY CAST SEGMENTS

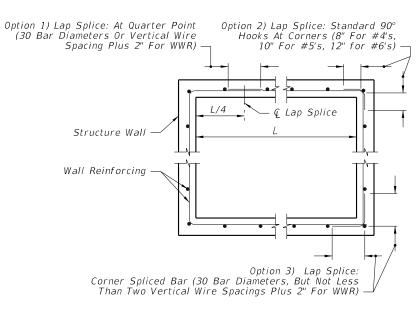
NOTE: h may be less than 6" when approved by the Engineer, but not for inlet segments at finish grade elevation

COMPARATIVE SIDE VIEWS MINIMUM DIMENSIONS FOR BOX AND RISER SEGMENTS



(NOTE: NOT APPLICABLE AROUND MANHOLE AND RISER OPENINGS)

REBAR STRAIGHT END EMBEDMENT FOR TOP AND BOTTOM SLABS



WALL REINFORCING SPLICE DETAILS

REVISION 07/01/15

Cast Joint



			EXAMPLE TABLE	OF EQU	IVALENT STEEL	AREA		
SCHEDULE	GRADE 60 REINFORCING BAR		EQUIVALENT GRADE 40 REINFORCING BAR		EQUIVALENT 65 KSI SMOOTH WELDED WIRE REINFORCEMENT		EQUIVALENT 70 KSI DEFORMED WELDED WIRE REINFORCEMENT	
	Bar Size & Spacing	Steel Area (in²/ft)	Bar Size & Spacing	Min. Steel Area (in²/ft)	Style Designation	Min. Steel Area (in²/ft)	Style Designation	Min. Steel Area (in²/ft)
А	#3 @ 6½" Ctrs. #4 @ 12" Ctrs.	0.20	#3 @ 4½" Ctrs. #4 @ 8" Ctrs. #5 @ 12" Ctrs.	0.30	3"x3"-W4.6xW4.6 4"x4"-W6.2xW6.2 6"x6"-W9.2xW9.2	0.1846	3"x3"-D4.3xD4.3 4"x4"-D5.7xD5.7 6"x6"-D8.6xD8.6	0.1714
В	#3 @ 5½" Ctrs. #4 @ 10" Ctrs.	0.24	#3 @ 3½" Ctrs. #4 @ 6½" Ctrs. #5 @ 10" Ctrs.	0.36	3"x3"-W5.5xW5.5 4"x4"-W7.4xW7.4 6"x6"-W11.1xW11.1	0.2215	3"x3"-D5.1xD5.1 4"x4"-D6.9xD6.9 6"x6"-D10.3xD10.3	0.2057
Special 1	#3 @ 5" Ctrs #4 @ 9" Ctrs.	0.267	#3 @ 3" Ctrs. #4 @ 6" Ctrs. #5 @ 9" Ctrs.	0.40	3"x3"-W6.2xW6.2 4"x4"-W8.2xW8.2 6"x6"-W12.3xW12.3	0.2465	3"x3"-D5.7xD5.7 4"x4"-D7.6xD7.6 6"x6"-D11.4xD11.4	0.2289
С	#3 @ 3½" Ctrs. #4 @ 6½" Ctrs. #5 @ 10" Ctrs.	0.37	#4 @ 4" Ctrs. #5 @ 6½" Ctrs. #6 @ 9½" Ctrs.	0.555	3"x3"-W8.5xW8.5 4"x4"-W11.4xW11.4 6"x6"-W17.1xW17.1	0.3415	3"x3"-D7.9xD7.9 4"x4"-D10.6xD10.6 6"x6"-D15.9xD15.9	0.3171
D	#4 @ 4½" Ctrs. #5 @ 7" Ctrs. #6 @ 10" Ctrs.	0.53	#4 @ 3" Ctrs. #5 @ 4½" Ctrs. #6 @ 6½" Ctrs.	0.795	3"x3"-W12.2xW12.2 4"x4"-W16.3xW16.3 6"x6"-W24.5xW24.5	0.4892	3"x3"-D11.4xD11.4 4"x4"-D15.1xD15.1 6"x6"-D22.7xD22.7	0.4543
Е	#4 @ 3" Ctrs. #5 @ 5" Ctrs. #6 @ 7" Ctrs.	0.73	#5 @ 3½" Ctrs. #6 @ 4½" Ctrs. #7 @ 6½" Ctrs.	1.095	3"x3"-W16.8xW16.8 4"x4"-W22.5xW22.5 6"x6"-W33.7xW33.7	0.6738	3"x3"-D15.6xD15.6 4"x4"-D20.9xD20.9 6"x6"-D31.3xD31.3	0.6257
F	#5 @ 3½" Ctrs. #6 @ 5" Ctrs. #7 @ 7" Ctrs.	1.06	#6 @ 3" Ctrs. #7 @ 4½" Ctrs. #8 @ 6" Ctrs.	1.59	3"x3"-W24.5xW24.5 4"x4"-W32.6xW32.6 6"x6"-W48.9xW48.9	0.9785	3"x3"-D22.7xD22.7 4"x4"-D30.3xD30.3 6"x6"-D45.4xD45.4	0.9086
Special 2	#5 @ 3" Ctrs. #6 @ 4" Ctrs. #7 @ 5½" Ctrs.	1.24	#7 @ 4" Ctrs. #8 @ 5" Ctrs.	1.86	3"x3"-W28.6xW28.6 4"x4"-W38.2xW38.2 6"x6"-W57.2xW57.2	1.1446	3"x3"-D26.6xD26.6 4"x4"-D35.4xD35.4 6"x6"-D53.1xD53.1	1.0629
G	#6 @ 3½" Ctrs. #7 @ 5" Ctrs.	1.46	#7 @ 3" Ctrs. #8 @ 4" Ctrs.	2.19	3"x3"-W33.7xW33.7 4"x4"-W44.9xW44.9	1.3477	3"x3"-D31.3xD31.3 4"x4"-D41.7xD41.7	1.2514

GENERAL NOTES

- 1. For square or rectangular precast drainage structures, using either deformed or smooth WWR meeting the requirements of Specification Section 931, WWR shall be continuous around the box and lapped in accordance with Option 1 or 3 as shown in the Wall Reinforcing Splice Details.
- 2. Horizontal steel in the walls of rectangular structures shall be lap spliced in accordance with Option 1, 2 or 3 as shown in the Wall Reinforcing Splice Details.
- 3. Welding of splices and laps is permitted. The requirements and restrictions placed on welding in AASHTO M259 shall apply.
- 4. Rebar straight end embedment of peripheral reinforcement may be used in lieu of ACI standard hooks for top and bottom slabs except when hooks are specifically called for in the plans or standard drawings.
- 5. Concrete as specified in ASTM C478, (4000 psi) may be used in lieu of Class II concrete in precast items manufactured in plants which meet the requirements in accordance with Specification Section 449.
- 6. Precast opening for pipe shall be the pipe OD plus 6" (± 2" tolerance). Mortar used to seal the pipe into the opening will be of such a mix that shrinkage will not cause leakage into or out of the structure. Dry-pack mortar may be used in lieu of brick and mortar construction to seal openings less than $2\frac{1}{2}$ " wide.
- 7. For pay item purposes, the height used to determine if a drainage structure is greater than 10 feet shall be computed using: A, the elevation of the top of the manhole lid,
 - B. the grate elevation or the theoretical gutter grade elevation of an inlet, or
 - C. the outside top elevation of a junction box less the flow line elevation of the lowest pipe or to top of sump floor.

NOTES FOR PRECAST OPTIONS AND EQUIVALENT REINFORCEMENT SUBSTITUTION

- 1. Details for optional precast inlet construction up to depths of 15' are shown on the inlet indexes.
- 2. When precast units are used in conjunction with Alt. "B" Structure Bottoms, Index No. 200, the interior dimensions of an Alt. "B" Bottom can be adjusted to reflect these inlet interior dimensions
- 3. Concrete which meets the requirements of ASTM C478 or Class IV must be used for precast structures constructed with 6" wall or slab thickness.
- 4. Reinforcement can be either deformed bar reinforcement or welded wire reinforcement. Bar reinforcement other than 60 ksi may be used, however only two grades are recognized; Grade 40 and Grade 60. Smooth welded wire reinforcement, will be recognized as having a design strength of 65 ksi and deformed welded wire reinforcement will be recognized as having a design strength of 70 ksi. The area of reinforcement required may be adjusted in accordance with the Equivalent Steel Area Table provided. For bars and spacings not given, the steel area required can be determined by the following equations:

Grade 40 Steel Area =
$$A_540 = \frac{60}{40} \times A_560$$

Smooth Welded Wire Reinforcement Steel Area = $A_565 = \underline{60} \times A_560$

Deformed Welded Wire Reinforcement Steel Area = $A_c70 = \underline{60} \times A_c60$

When a reduced area of reinforcement is provided, any maximum bar spacing shown must also be reduced as determined by the following equations, unless otherwise shown:

Max. Grade 40 Bar Spacing = Grade 60 Bar Spacing

Max. Smooth Welded Wire Spacing = Grade 60 Bar Spacing x 0.86

Max. Deformed Welded Wire Spacing = Grade 60 Bar Spacing x 0.74

When an increased area of reinforcing is provided, then the maximum bar spacing may be increased by the squared ratio of increased steel area, but not to exceed 12":

Max. Bar Spacing Provided \leq Max. Bar Spacing Required \times $\left(\frac{\text{Steel Area Provided}}{\text{Min. Steel Area Required}}\right)$

In no case will reinforcement with wires smaller than W3.1 or D4.0, or spacings greater than 8" be permitted. Bar reinforcement shall show the minimum yield designation grade mark or either the number 60 or one (1) grade mark line to be acceptable at the higher value. Maximum bar spacing shall not be greater than two (2) times the slab thickness with a maximum spacing of 12" or three (3) times the wall thickness, with a maximum spacing of 18" for vertical bars and 12" for horizontal bars. Wires smaller than W3.1 or D4.0 are permitted in the walls of ASTM C 478 round structure bottoms and round risers.

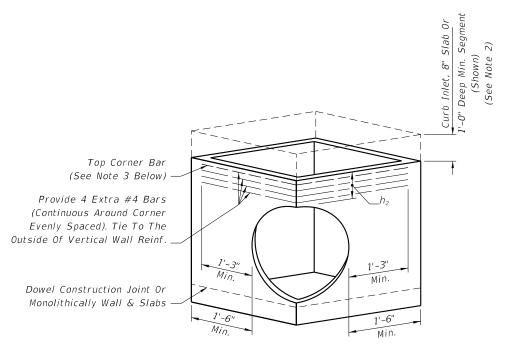
5. Fiber-reinforced concrete may be substituted for conventional steel reinforcement in accordance with the Structures Design Guidelines. Shop drawings corresponding to an approved fiber-reinforced concrete mix design must be submitted for approval to the State Drainage Engineer.

11/01/16

SHEET

NO.

4 of 5



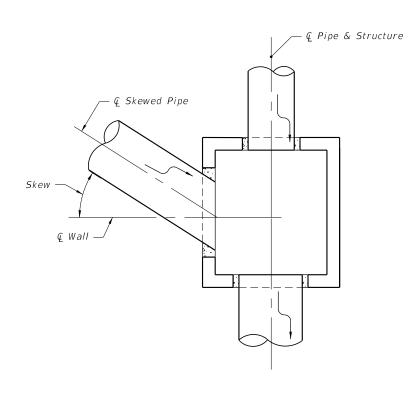
 $h_2 \ge 1'-0''$ (See Notes 2 and 3 Below)

DESIGNER NOTE: Use only when round structures are not practical, engineer of record approval required.

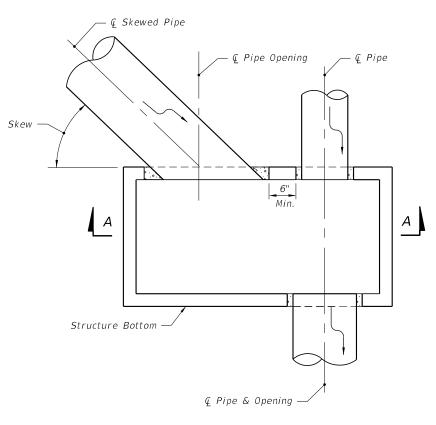
PICTORIAL VIEW

- NOTE: 1. Submit Shop Drawings of corner openings for approval by the Engineer of Record.
 - 2. h_2 may be less than 1'-0" when a minimum 1'-0" deep segment, 8" slab or curb inlet is provided above the corner opening.
 - 3. For inlet segments at finish grade elevation substitute a #8 Bar for the top corner bar when $1'-0'' \le h_2 < 2'-0''$.

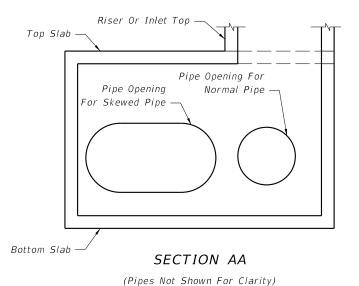
RECTANGULAR SEGMENT WITH PIPE OPENING AT CORNER



PLAN VIEW FOR SKEWS ≤ 45° (Not Centered)



PLAN VIEW FOR SKEWS > 45° (Not Centered)



DETAILS FOR SKEWED PIPES IN RECTANGULAR STRUCTURES

DESCRIPTION: REVISION

07/01/12