

END 2

BEAM NOTES

- 1. All bar dimensions are out-to-out.
- 2. Place one (1) Bar 5K or 5Z at each location as detailed alternating the direction of the ends for each bar (see "ELEVATION AT END OF BEAM", Index Nos. 20036, 20045, 20054, 20063, 20072, 20078, 20084 and 20096).
- 3. Strands N shall be ASTM A416, Grade 270, seven-wire strands 🐉 Ø or larger, stressed to 10,000 lbs. each.
- 4. For beams with ends not to be encased in permanent concrete diaphragms, after detensioning cut wedge to recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See STRAND CUTTING AND PROTECTING DETAIL on Sheet 2.
- 5. For beams with ends not to be encased in permanent concrete diaphragms, protect end of recessed strands in accordance with Specification Section 450.
- 6. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 7. At the Contractor's option, welded deformed wire reinforcement may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details for each beam size. Welded deformed wire reinforcement shall meet requirements of Specification Section 931.
- 8. Safety Line Anchorage Devices or sleeves are required and permitted in the top flange only to accommodate fall protection systems used during construction. See shop drawings for details and spacing of any required embedments.
- 9. For beams with skewed end conditions, the end reinforcement, defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM", shall be placed parallel to the skewed end of the beam. Bars 3D3, 5K and 4M3 located beyond the limits of Bars 3C shall be placed perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the Number Required on the "BILL OF REINFORCING STEEL". For placement locations, see "SKEWED BEAM END DETAILS". Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the "BENDING DIAGRAM" for skewed end conditions.
- 10. Placement of Bars 3C1, 3D1 and 4M1 correspond to END 1, and Bars 3C2, 3D2 and 4M2 correspond to END 2. END 1 and END 2 are shown on the beam "ELEVATION".
- 11. For Beams with vertically beveled end conditions, place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For welded deformed wire reinforcement, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
- 12. For beams with skewed end conditions, welded deformed wire reinforcement shall not be used for end reinforcement (Bars 3D1, 3D2, 4M1 and 4M2)
- 13. Bars 5K and 5Z shall be placed and tied to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables in Structures Plans). At the Contractor's option the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands. For welded deformed wire reinforcement, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 14. At the Contractor's option, Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 15. For referenced Dimensions, Angles and Case Numbers, see the Table of Beam Variables in Structures Plans.

CASE 3

(Special Orientation for Widenings)

END 1

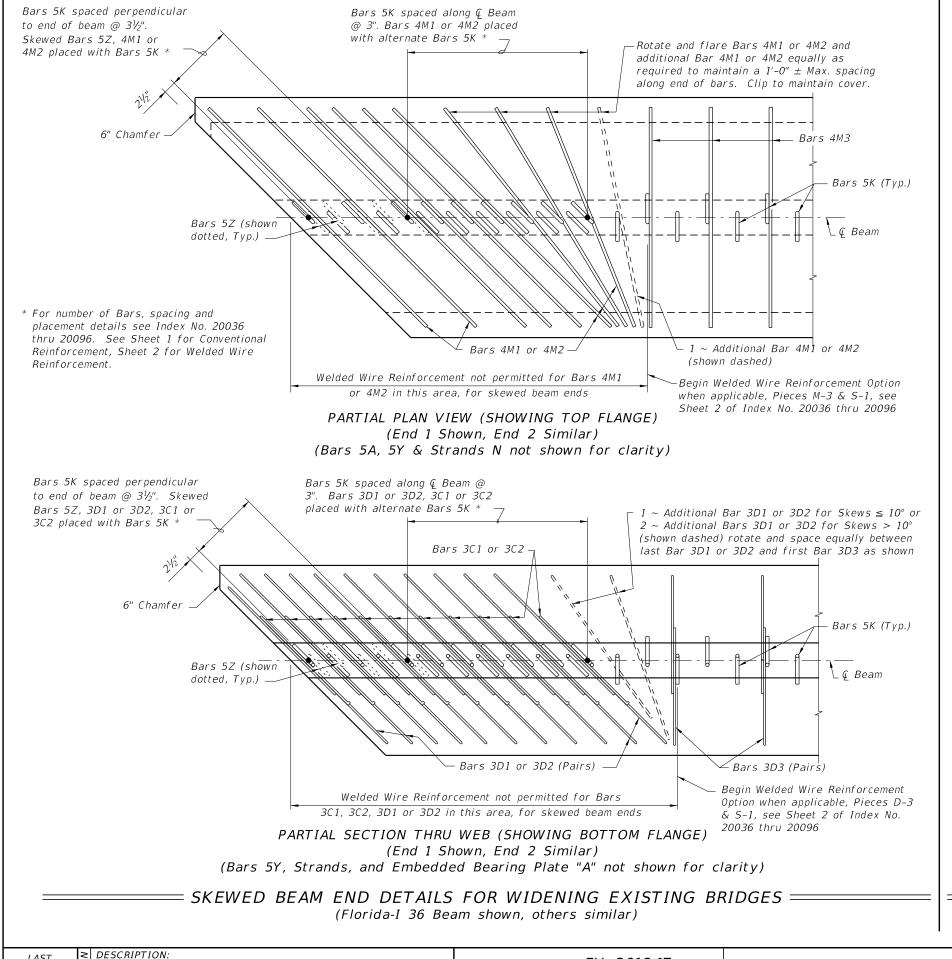
SCHEMATIC END ELEVATIONS OF BEAMS

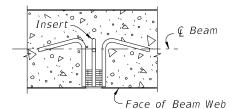
CONDITION 3

(Showing Vertical Bevel of Beam End)

DESCRIPTION: REVISION 07/01/14







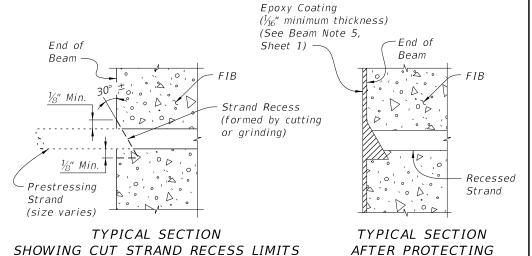
PLAN SECTION THRU BEAM WEB AT INSERT FOR DIAPHRAGM REINFORCING

(When Intermediate Diaphragms are Required by Design)

INSERT NOTES

- 1. Provide 1" Ø, zinc-electroplated, ferrule wing nut or coil inserts, UNC threads, 1/0 minimum gage wire, not more than 4" in depth with a minimum ultimate tensile strength of 11,400 lbs. in 4,000 psi concrete.
- 2. If inserts are needed on both sides (faces) of beam webs, an assembly as long as the thickness of the beam web, consisting of two (2) ferrule or coil inserts attached by two (2) or more struts may be utilized. The connecting struts shall have a minimum ultimate tensile strength of 11,400 lbs.
- 3. Inserts for diaphragm reinforcing are required at each end of each intermediate diaphragm shown on the Beam Framing Plan and may be required at the end of the beams when end diaphragms are shown. See Superstructure and Beam Framing Plans for longitudinal location of inserts for each face of beam.

==== INSERT DETAIL ====



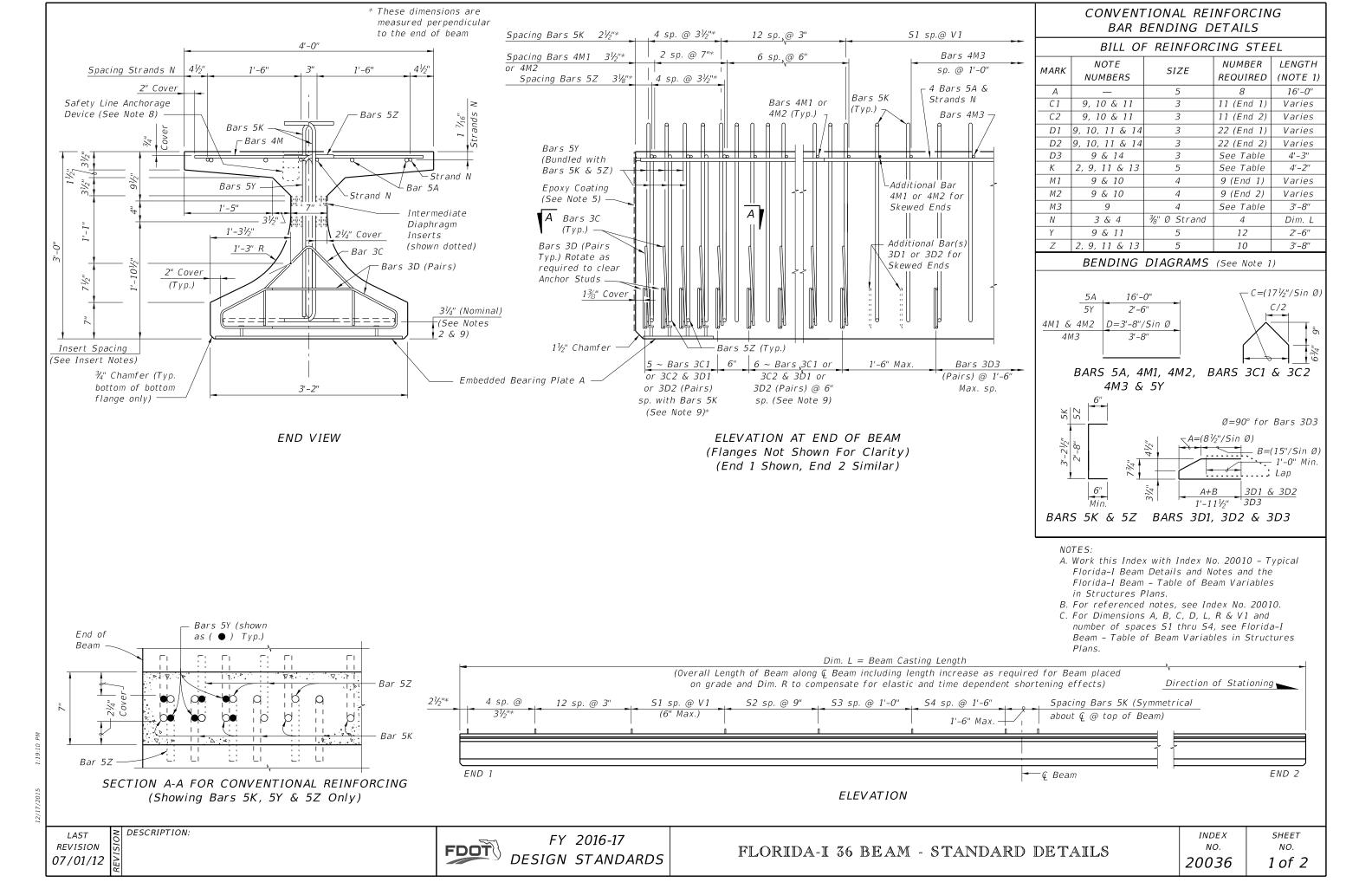
ig|==== STRAND CUTTING AND PROTECTING DETAIL ====

REVISION 07/01/14

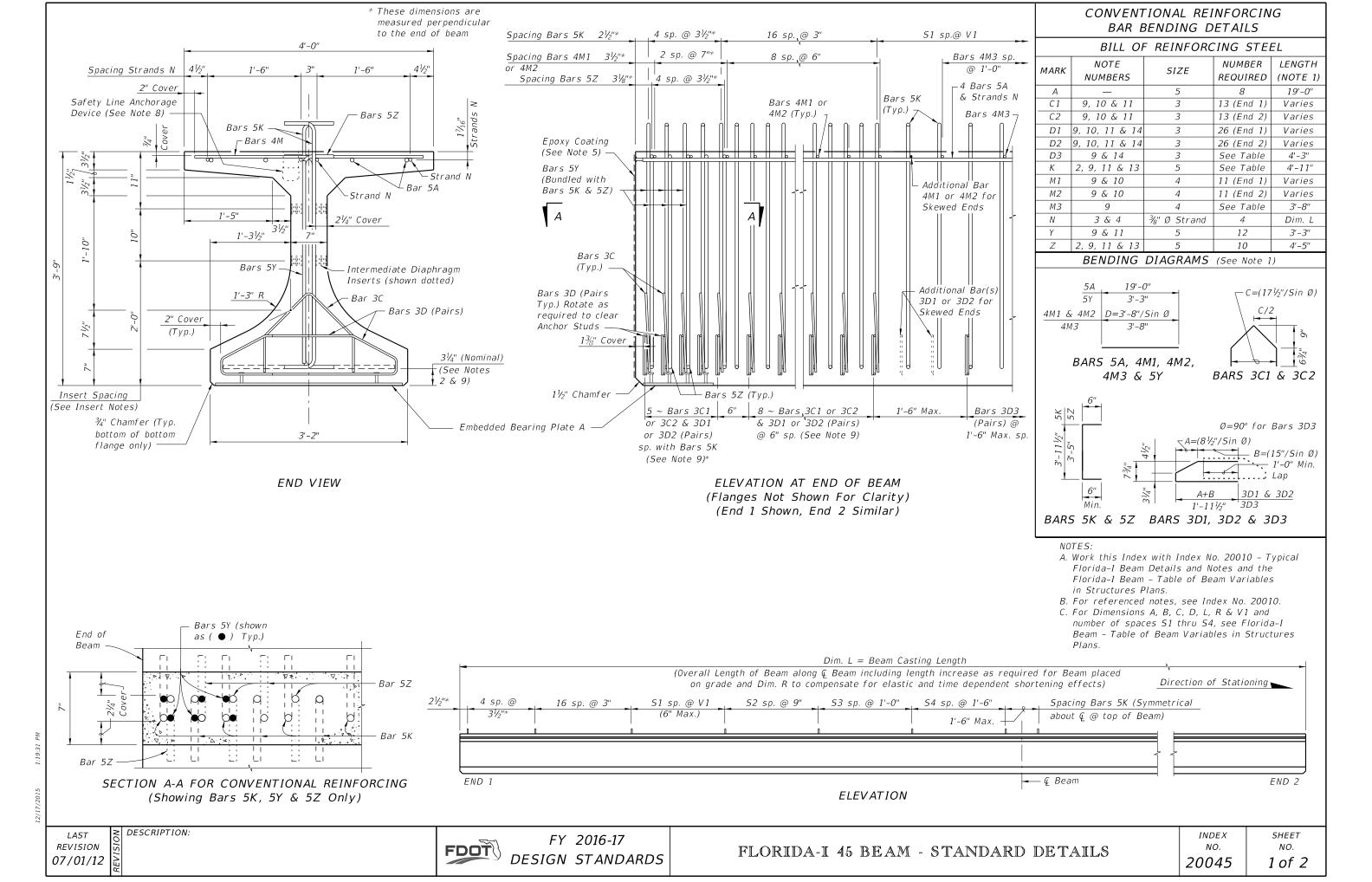
FY 2016-17 DESIGN STANDARDS DETAILS AND NOTES

INDEX NO. 20010

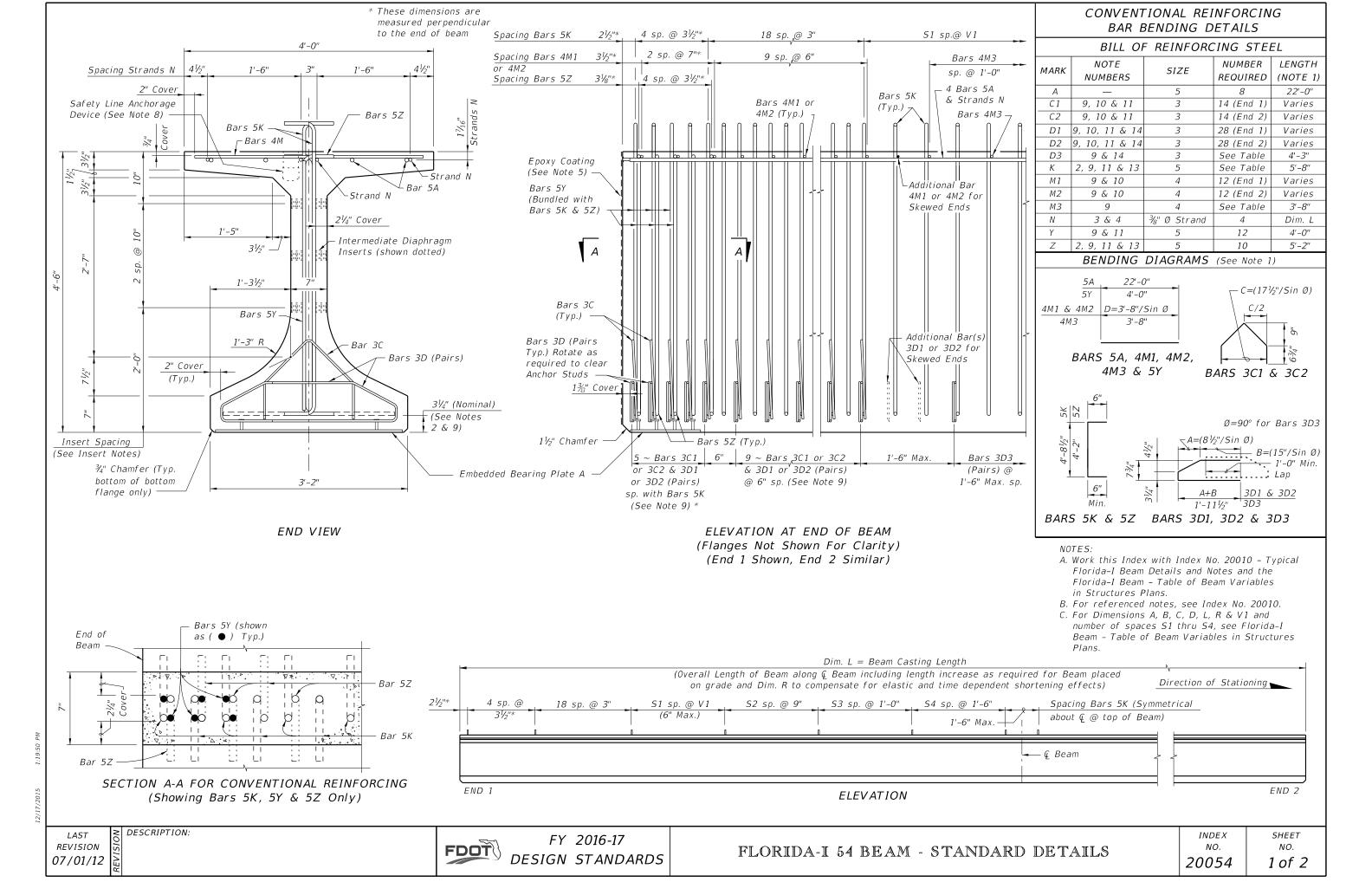
SHEET NO. 2 of 2



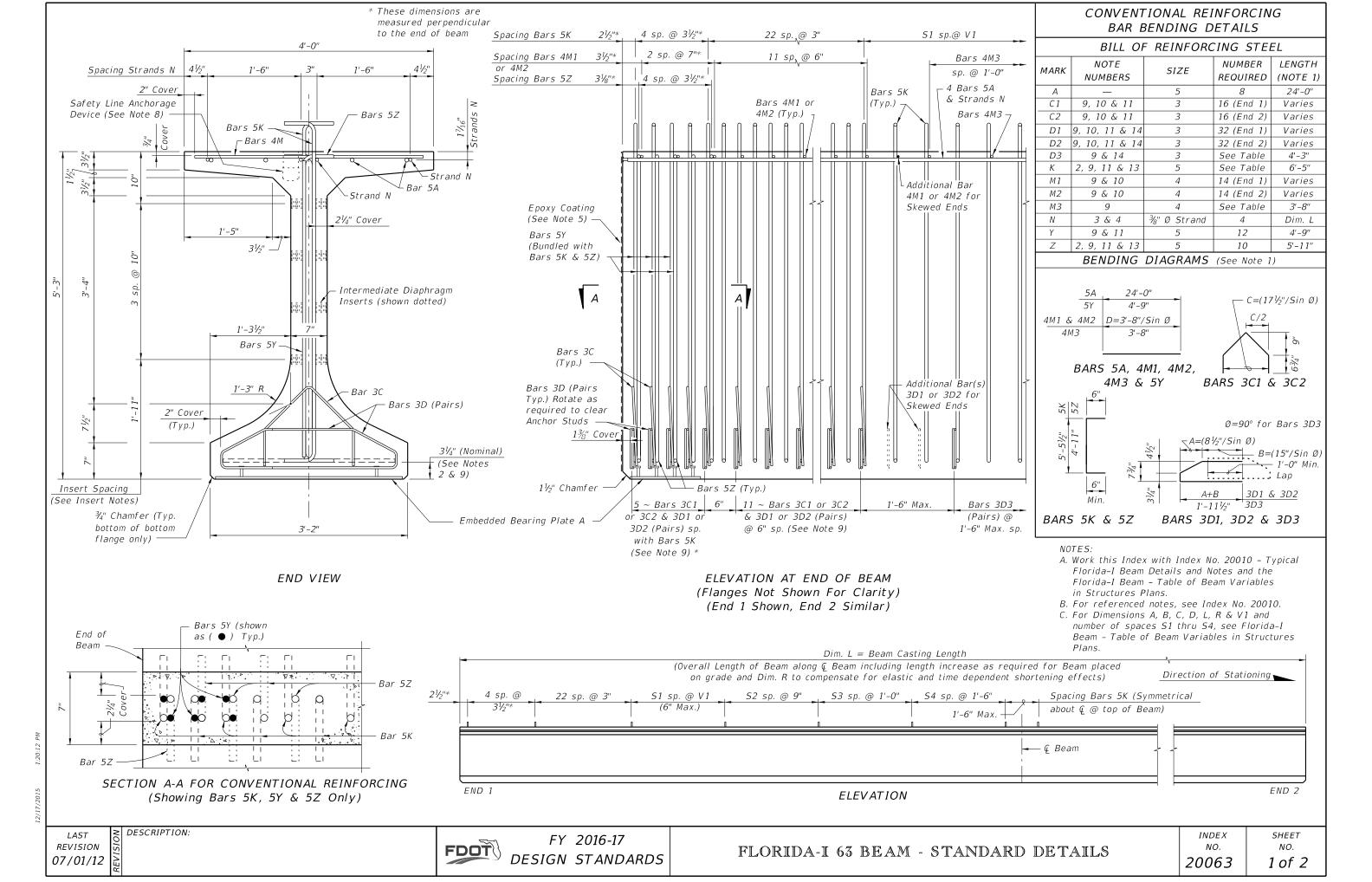
ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS 9 ~ D16's @ 6" sp. = 4'-0" D16's @ 1'-0" sp. Bars 5Y (12 Required) -Wires D31 (shown (shown as (●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max Optional W6.4 -W6.4 1'-0" Max. Beam — Optional W6.4 - W6.4 W6.4-D16-Optional W6.4 D16 1" extension W6.4 (Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) PLAN VIEW PLAN VIEW SECTION A-A PIECES M PIECE M-1 PIECE M-3 - Match spacing FOR WELDED WIRE REINFORCEMENT 5" ± Piece M-1 ties END VIEW of adjacent (2 Required) (2 Required) to Piece K-2 Piece S-1, S-2, Pieces S (Single Mat) Tied 6 ~ D25's (FF) S-3 or S-4 21/5" to Strands at © Beam) S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam -Pieces M-3 6 ~ D25's (BF) S2 ~ D25's @ 9" sp. (Piece S-2) @ 6'' = 2'-6'' $3\frac{1}{2}$ " sp. = S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 2" Cover Varies 9" Max. 25/8" Cover (Typ.)(Typ.)(Typ.)(Typ.)3¾" ± PARTIAL SECTION AT CENTER BEAM W12.4 (Piece K-1) Pieces K (Pairs) W10 (Pieces K-2 & S) Pieces M-1 W10) or D31 (Piece K-1 (W12.4 2" Cover D25 (Pieces K-2 & S) -21/4" Cover (Typ.)(2" Min.) Ø Wires (_ ⊊ Beam (WWR -W12.4 (Piece K-1) Symmetrical) Pieces D W10 (Pieces K-2 & S) Piece Cover (Pairs) 1" extension (Typ.) PARTIAL BEAM END VIEW PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 (Conventional Reinforcing Bars A, C, Y END VIEW (Aligned EF) (FF Shown Solid, (2 Required Each Piece) and Strands not Shown for Clarity) (4 Required ~ BF Shown Dashed) Piece D-1 ties to Piece K-1 2 Pairs) (4 Required) NOTES: a. See Sheet 1 for placement details & Table of Beam 5 ~ D11's @ $6 \sim D11's @ 6'' sp. = 2'-6''$ D11's @ 1'-6" spaces 1'-3" Variables in Structures Plans for variables S1, S2, S3, $3\frac{1}{2}$ " sp. = Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, - 1" extension (Typ.) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 D-1, D-2 & D-3) EF = Each FaceSkew Details and Note 9 for placement details. Shift PIECE D-1 PIECE D-2 PIECE D-3 FF = Front Face Pieces K & Bars 5Y to accommodate skewed end PIECES D BF = Back Faceconditions and align with Bars C and D. (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 36 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20036 2 of 2



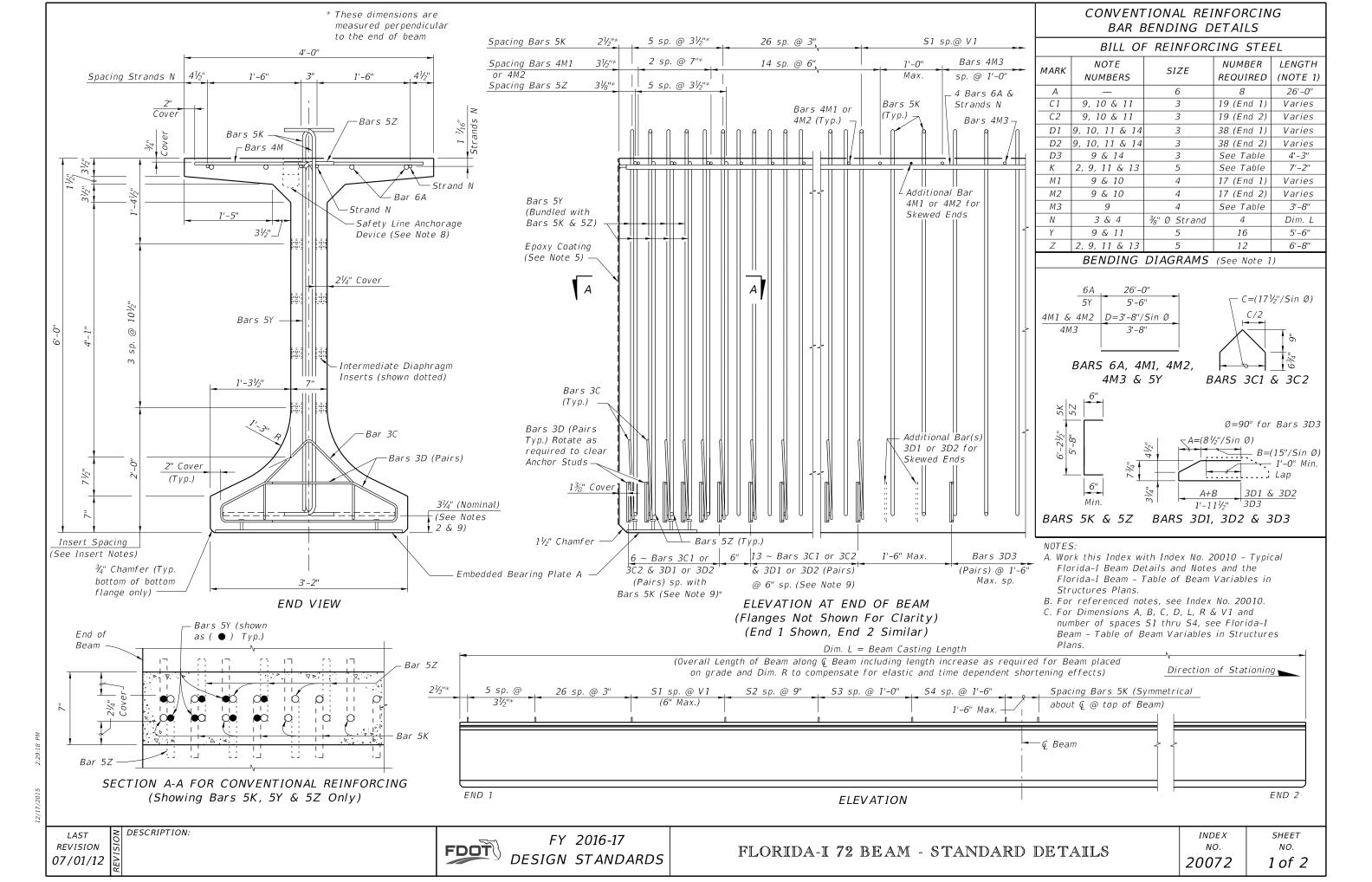
ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (12 Required) -Wires D31 (shown $11 \sim D16's @ 6'' sp. = 5'-0''$ D16's @ 1'-0" sp. (shown as (●) Typ.) as(O) Typ.) End of Varies Varies 9" Max. Beam Optional W6.4 W6.4 1'-0" Max. Optional W6.4 _ W6.4 W6.4 - D16 Optional W6.4 D16 -16.......... 1" extension W6.4-(Typ.)21/4" Cover −Pieces K-1 Pieces K-2 (Offset) SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT PIECE M-3 PIECES M PIECE M-1 $5'' \pm | Piece M-1 ties$ - Match spacing of END VIEW (2 Required) (2 Required) Pieces S (Single Mat) Tied adjacent Piece S-1, to Piece K-2to Strands at @ Beam) S-2, S-3 or S-4 $8 \sim D25's (FF) @ 6'' = 3'-6"$ Pieces M-3 S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam — 5 ~ D31's @ $8 \sim D25's$ (BF) @ 6'' = 3'-6''S2 ~ D25's @ 9" sp. (Piece S-2) S3 ~ D25's @ 1'-0" sp. (Piece S-3) $3\frac{1}{2}$ " sp. = 2" Cover S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 25/8" Cover (Typ.) $3\frac{3}{4}$ " ± Varies 9" Max (Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) Pieces M-1 W12.4 (Piece K-1) W10 (Pieces K-2 & S) W10) - D31 (Piece K-1) 2" Cover D25 (Pieces K-2 & S) 21/4" Cover 01 (Typ.)(2" Min.) (W12.4 ⊊ Geam (WWR -W12.4 (Piece K-1) Symmetrical) W10 (Pieces K-2 & S) Pieces D Cover (Pairs) 1" extension (Typ.) PIECES K & S PIECE K-2 PIECE S-1, S-2, S-3 or S-4 PIECE K-1 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required Each Piece) PARTIAL BEAM END VIEW (4 Required (4 Required) (Conventional Reinforcing Bars A, C, Y Piece D-1 ties and Strands not Shown for Clarity) ~ 2 Pairs) to Piece K-1 -NOTES: 5 ~ D11's @ $8 \sim D11's @ 6'' sp. = 3'-6''$ D11's @ 1'-6" spaces a. See Sheet 1 for placement details & Table of Beam Variables $3\frac{1}{2}$ " sp. = in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 1'-6" Max. Varies 9" Max 1'-2" -W4.4 b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" └ D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, —- | → 1" extension (Typ.) LEGEND: 1'-111½" D-1, D-2 & D-3) D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift FF = Front Face PIECE D-1 PIECE D-2 PIECE D-3 PIECES D Pieces K & Bars 5Y to accommodate skewed end BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) conditions and align with Bars C and D. END VIEW DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 45 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20045 2 of 2



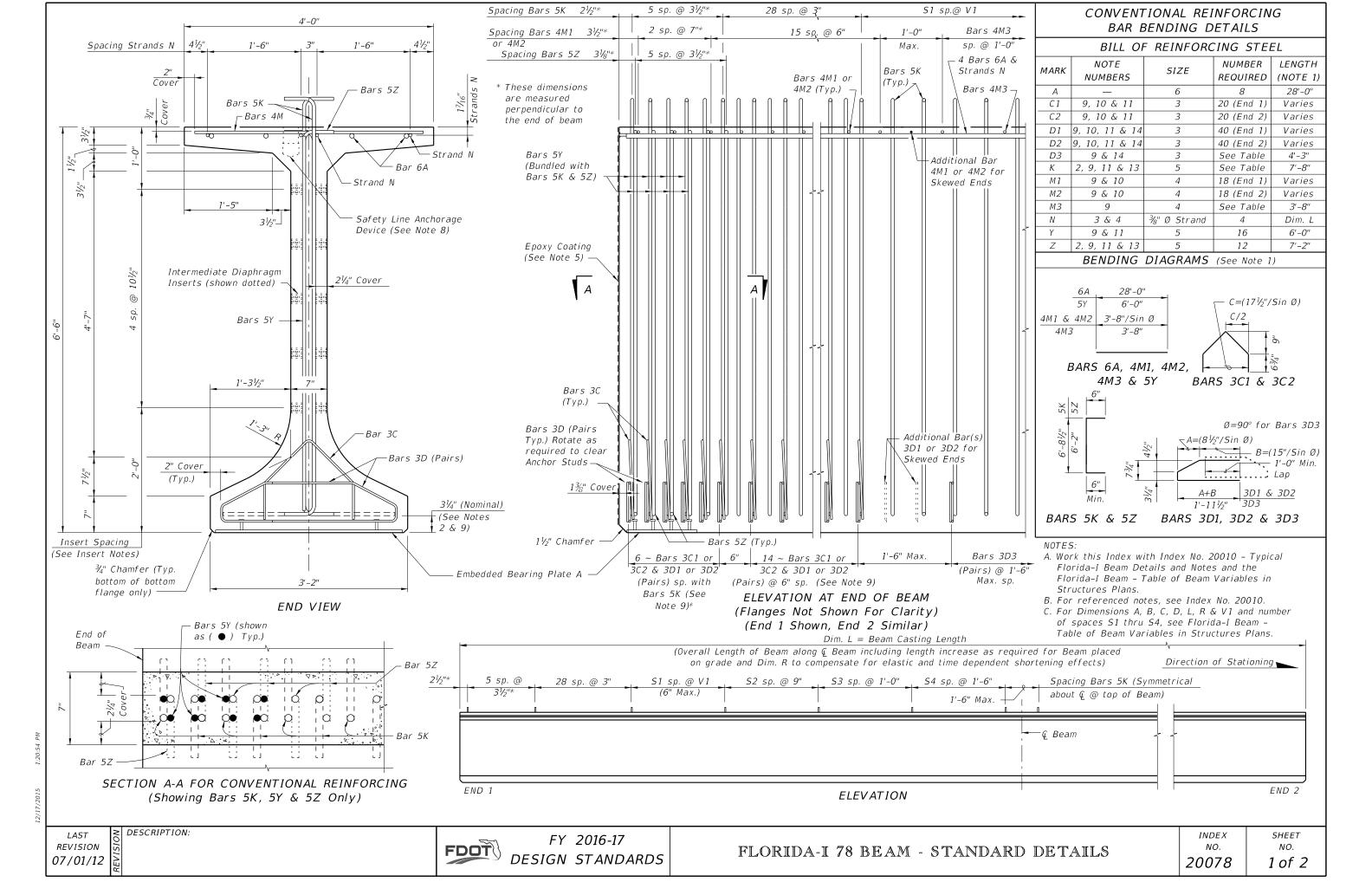
ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (12 Required) -Wires D31 (shown $12 \sim D16's @ 6'' sp. = 5'-6''$ D16's @ 1'-0" sp. as(O) Typ.) End of Varies Varies 9" Max. Optional W6.4 Beam Optional W6.4 - W6.4 1'-0" Max. W6.4 D16 Optional W6.4 D16 -*!* L = = = = | L = = = = = = = = = 1" extension W6.4-(Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT PIECE M-1 PIECES M PIECE M-3 Piece M-1 ties Match spacing of (2 Required) END VIEW (2 Required) Pieces S (Single Mat) Tied adjacent Piece S-1, to Piece K-2 S-2, S-3 or S-4 to Strands at & Beam) $9 \sim D25's (FF) @ 6'' = 4'-0''$ S1 ~ D25's @ V1 sp. (Piece S-1 shown) Pieces M-3 End of Beam — 5 ~ D31's @ $9 \sim D25's (BF) @ 6'' = 4'-0''$ S2 ~ D25's @ 9" sp. (Piece S-2) S3 ~ D25's @ 1'-0" sp. (Piece S-3) $3\frac{1}{2}$ " sp. = S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 1'-2" 3" Offset 25/8" Cover (Typ.)Varies 9" Max. 3¾" ± (Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) -8¾" D31 (Piece K-1) D25 (Pieces K-2 & S) Or 2" Cover (W12.4 (Typ.)⊈ Beam (WWR S -W12.4 (Piece K-1) Wires Symmetrical) Ø W10 (Pieces K-2 & S) Pieces D Cover (Pairs) 1" extension (Typ.) PIECES K & S PIECE S-1, S-2, S-3 or S-4 PIECE K-1 PIECE K-2 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required Each Piece) PARTIAL BEAM END VIEW (4 Required (4 Required) (Conventional Reinforcing Bars A, C, Y Piece D-1 ties and Strands not Shown for Clarity) ~ 2 Pairs) to Piece K-1 -NOTES: 5 ~ D11's @ 9 ~ D11's @ 6" sp. = 4'-0" D11's @ 1'-6" spaces a. See Sheet 1 for placement details & Table of Beam $3\frac{1}{2}$ " sp. = Variables in Structures Plans for variables S1, S2, S3, Varies 1'-6" Max. Varies 9" Max 54 & V1. 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" D11 (Typ. Pieces 💶 🗕 1" extension (Typ.) LEGEND: M-1 shall not be used; Conventional Reinforcement Bars D1, 1'-111/2" D-1, D-2 & D-3) EF = Each FaceD2, C1, C2, M1 & M2 shall be used. See Index No. 20010 FF = Front Face Skew Details and Note 9 for placement details. Shift PIECE D-2 PIECE D-1 PIECE D-3 PIECES D Pieces K & Bars 5Y to accommodate skewed end BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW conditions and align with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 54 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20054 2 of 2



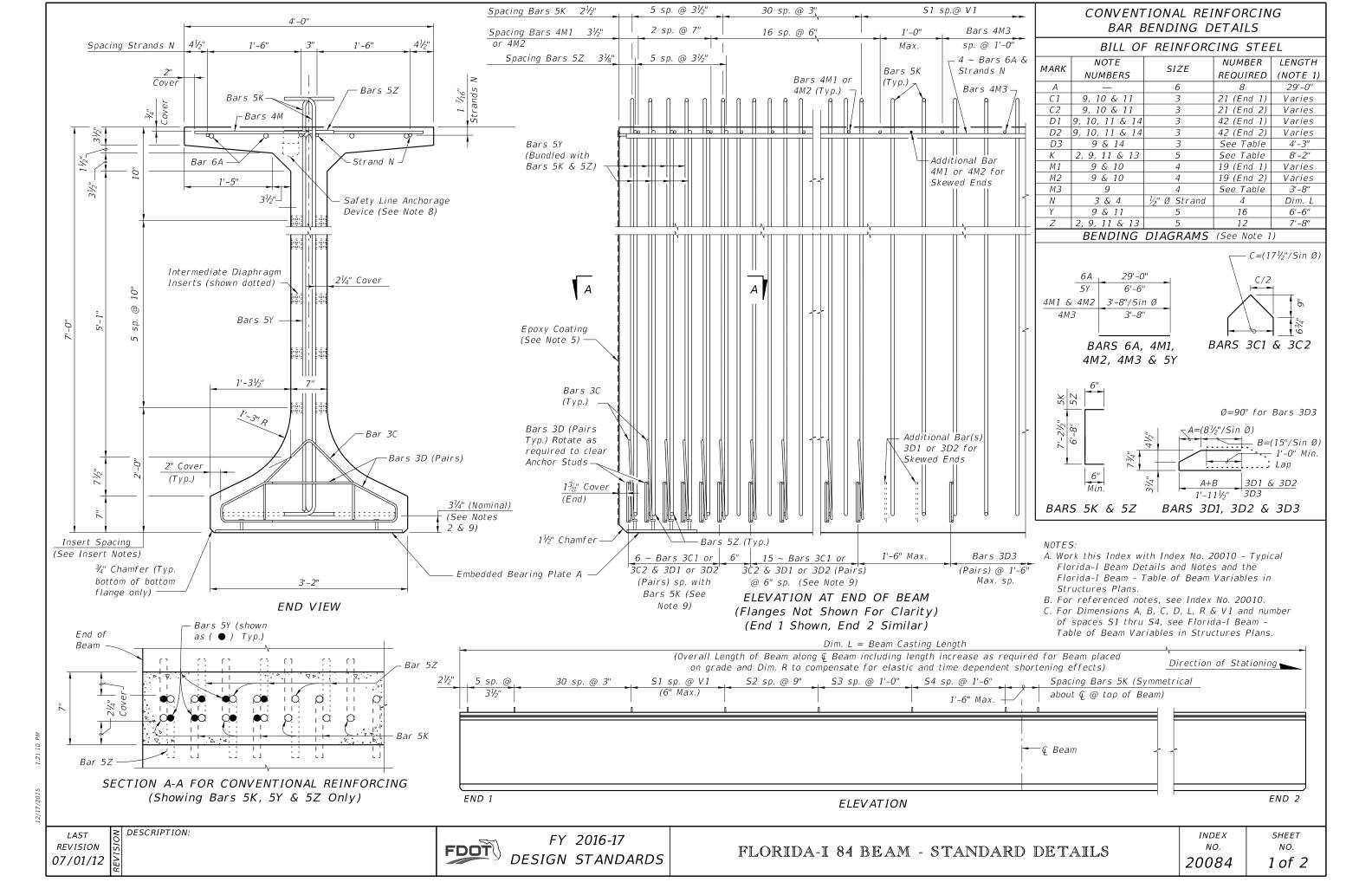
ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (12 Required) -Wires D31 (shown $14 \sim D16's @ 6" sp. = 6'-6"$ D16's @ 1'-0" sp. (shown as (●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max. Beam Optional W6.4 - W6.4 1'-0" Max. - Optional W6.4 W6.4 W6.4 D16-Optional W6.4 D16 -. | | 1 - - - - -. W6.4 21/4" Cover Pieces K-1 Pieces K-2 1" extension (Offset) -(Typ.) -SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT PIECE M-3 PIECES M PIECE M-1 5" ± Piece M−1 ties Match spacing of END VIEW (2 Required) (2 Required) Pieces S (Single Mat) Tied adjacent Piece S-1, to Piece K-2 S-2, S-3 or S-4 to Strands at (Beam) $11 \sim D25's (FF) @ 6'' = 5'-0''$ S1 ~ D25's @ V1 sp. (Piece S-1 shown) Pieces M-3 End of Beam — D31's @ $11 \sim D25's (BF) @ 6'' = 5'-0''$ S2 ~ D25's @ 9" sp. (Piece S-2) 53 ~ D25's @ 1'-0" sp. (Piece S-3) $3\frac{1}{2}$ " sp. = S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 1'-2" 3" Offset 6" 6" 25/8" Cover (Typ.) Varies 9" Max. 3¾" ± (Typ.)(Typ.)(Typ.) PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) → D31 (Piece K-1) \coprod D25 (Pieces K-2 & S) Or 21/4" Cover Wires (W12.4 W12.4 (Piece K-1) δ Symmetrical) W10 (Pieces K-2 & S) Pieces D Piece Cover Cross (Pairs) - 1" extension (Typ.) PIECES K & S PIECE K-2 PIECE K-1 PIECE S-1, S-2, S-3 or S-4 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required Each Piece) PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, C, Y (4 Required (4 Required) Piece D-1 ties and Strands not Shown for Clarity) ~ 2 Pairs) to Piece K-1 -NOTES: $5 \sim D11's$ @ 1'-3" $11 \sim D11's @ 6'' sp. = 5'-0''$ D11's @ 1'-6" spaces a. See Sheet 1 for placement details & Table of Beam Variables $3\frac{1}{2}$ " sp. = in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & ∠ D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, → | 1" extension (Typ.) LEGEND: 1'-111/2" D-1, D-2 & D-3) D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift Pieces K FF = Front Face PIECE D-1 PIECE D-2 PIECE D-3 PIECES D & Bars 5Y to accommodate skewed end conditions and align BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) **END VIEW** with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 63 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20063 2 of 2



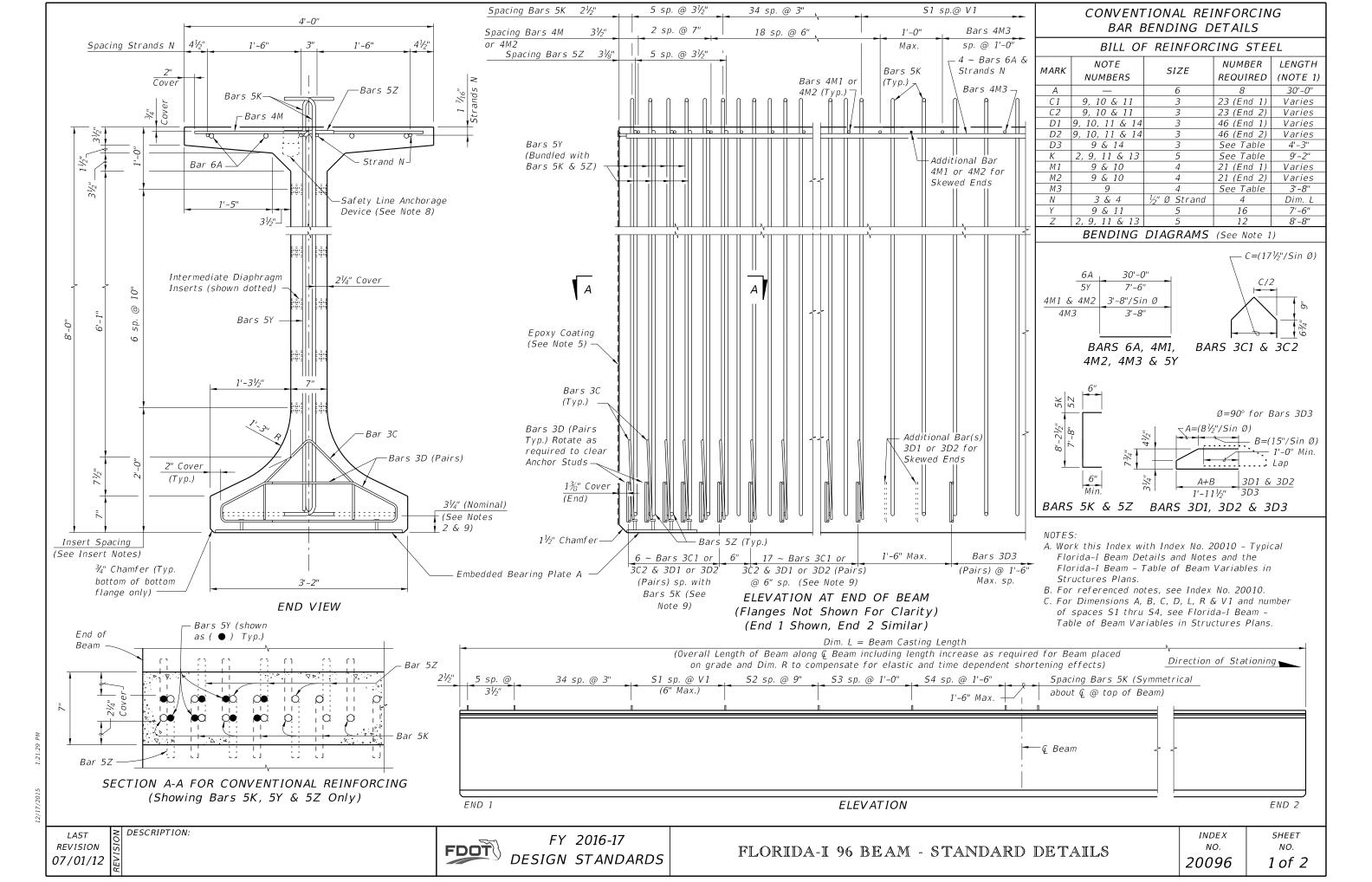
ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (16 Required) -Wires D31 (shown 17 ~ D16's @ 6" sp. = 8'-0" D16's @ 1'-0" sp. as(O) Typ.) End of Varies Varies 9" Max. Beam Optional W6.4 W6.4 1'-0" Max. Optional W6.4 - W6.4 W6.4 D16-Optional W6.4 D16 -. W6.4 21/4" Cover Pieces K-1 Pieces K-2 (Offset) 1" extension (Typ.)SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT 5½" ± PIECES M PIECE M-1 PIECE M-3 Piece M-1 ties Match spacing of END VIEW (2 Required) (2 Required) adjacent Piece S-1, Pieces S (Single Mat) Tied to Piece K-2 S-2, S-3 or S-4 to Strands at (Beam) $13 \sim D25's (FF) @ 6'' = 6'-0''$ S1 ~ D25's @ V1 sp. (Piece S-1 shown) Pieces M-3 End of Beam -6 ~ D31's @ 31/5" 13 ~ D25's (BF) @ 6" = 6'-0" S2 ~ D25's @ 9" sp. (Piece S-2) $sp. = 1'-5\frac{1}{2}"$ S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 3" Offset 25/8" Cover (Typ.) Varies 9" Max. 3¾" ± (Typ.)(Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) ~W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) **→** D31 (Piece K-1) __,∐__D25 (Pieces K-2 & S) Wires (W12.4 or 21/4" Cover -W12.4 (Piece K-1) δ W10 (Pieces K-2 & S) Symmetrical) Piece K Pieces D Cover (Pairs) - 1" extension (Typ.) PIECE K-2 PIECE S-1, S-2, S-3 or S-4 PIECES K & S PIECE K-1 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required ~ Each Piece) PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, C, Y (4 Required (4 Required) Piece D-1 ties and Strands not Shown for Clarity) ~ 2 Pairs) to Piece K-1 -NOTES: 6 ~ D11's @ 3½" 1'-3" 13 ~ D11's @ 6" sp. = 6'-0" D11's @ 1'-6"_spaces a. See Sheet 1 for placement details & Table of Beam Variables $sp. = 1'-5\frac{1}{2}''$ Varies 1'-6" Max. in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 9" Max - W4.4 b. Place Conventional Reinforcement Bars 6A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, 1" extension (Typ.) D-1, D-2 & D-3) LEGEND: 1'-111½" D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift Pieces K FF = Front Face PIECE D-1 PIECE D-2 PIECE D-3 PIECES D & Bars 5Y to accommodate skewed end conditions and align BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 72 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20072 2 of 2



ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (16 Required) -Wires D31 (shown $18 \sim D16's @ 6'' sp. = 8'-6'$ D16's @ 1'-0" sp. as(O) Typ.) End of Varies Varies 9" Max. Beam - Optional W6.4 W6.4 1'-0" Max. - Optional W6.4 - W6.4 W6.4 D16-Optional W6.4 D16 -. © ₩6.4 21/4" Cover Pieces K-1 Pieces K-2 1" extension (Offset) (Tvp.)SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT 5½" ± PIECE M-1 PIECE M-3 PIECES M Piece M-1 tied Match spacing of END VIEW (2 Required) (2 Required) Pieces S (Single Mat) Tied ad jacent Piece S-1, to Piece K-2 S-2, S-3 or S-4 to Strands at (Beam) $14 \sim D25's (FF) @ 6'' = 6'-6''$ S1 ~ D25's @ V1 sp. (Piece S-1 shown) Pieces M-3 End of Beam -6 ~ D31's @ 31/5" $14 \sim D25's (BF) @ 6'' = 6'-6''$ S2 ~ D25's @ 9" sp. (Piece S-2) $sp. = 1'-5\frac{1}{5}$ " S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 3" Offset 6" 25/8" Cover (Typ.) Varies 9" Max. 3¾" ± (Typ.)(Typ.)(Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) ~W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) → D31 (Piece K-1) -8¾" D25 (Pieces K-2 & S) Wires (W12.4 or 21/4" Cover -W12.4 (Piece K−1) δ Symmetrical) W10 (Pieces K-2 & S) Pieces D Piece Cover (Pairs) Cross - 1" extension (Typ.) PIECE K-2 PIECES K & S PIECE K-1 PIECE S-1, S-2, S-3 or S-4 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required ~ Each Piece) PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, C, Y (4 Required (4 Required) Piece D-1 ties and Strands N not Shown for Clarity) ~ 2 Pairs) to Piece K-1 -NOTES: 6 ~ D11's @ 3½" 1'-3" 14 ~ D11's @ 6" sp. = 6'-6" D11's @ 1'-6"_spaces a. See Sheet 1 for placement details & Table of Beam Variables $sp. = 1'-5\frac{1}{2}''$ Varies 1'-6" Max. in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 9" Max ₩4.4 b. Place Conventional Reinforcement Bars 6A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" ∠ D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, - 1" extension (Typ.) D-1, D-2 & D-3) LEGEND: 1'-111½" D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift Pieces K FF = Front Face PIECE D-1 PIECE D-2 PIECE D-3 PIECES D & Bars 5Y to accommodate skewed end conditions and align BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 78 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20078 2 of 2



ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (16 Required) -Wires D31 (shown $19 \sim D16's @ 6'' sp. = 9'-0''$ D16's 1'-0" sp. as(O) Typ.) End of Varies Varies 9" Max. Beam Optional W6.4 W6.4 1'-0" Max. Optional W6.4 - W6.4 W6.4 D16-Optional W6.4 D16 -21/4" Cover W6.4 Pieces K-1 Pieces K-2 1" extension (Offset) (Typ.)SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT 5½" ± PIECE M-3 PIECES M PIECE M-1 Piece M-1 tied Match spacing of END VIEW (2 Required) (2 Required) adjacent Piece S-1, Pieces S (Single Mat) Tied to Piece K-2 S-2, S-3 or S-4 to Strands at @ Beam) 15 ~ D25's (FF) @ 6" = 7'-0" S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam -6 ~ D31's @ 31/3" $15 \sim D25's (BF) @ 6'' = 7'-0''$ S2 ~ D25's @ 9" sp. (Piece S-2) $sp. = 1'-5\frac{1}{2}"$ S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 3" Offset 25/8" Cover (Typ.) Varies 9" Max. 3¾" ± (Typ.)(Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) ~W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) → D31 (Piece K-1) D25 (Pieces K-2 & S) 2" Cover Wires (W12.4 or 21/4" Cover -W12.4 (Piece K−1) ___Q Beam (WWR Ø Symmetrical) W10 (Pieces K-2 & S) Pieces D Piece Cover (Pairs) Cross 1" extension (Typ.) PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required ~ Each Piece) PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, C, Y (4 Required ~ (4 Required) Piece D-1 ties and Strands N not Shown for Clarity) 2 Pairs) to Piece K-1— NOTES: $6 \sim D11's @ 3\frac{1}{2}"$ 1'-3" $15 \sim D11's @ 6'' sp. = 7'-0''$ D11's @ 1'-6" spaces a. See Sheet 1 for placement details & Table of Beam Variables $sp. = 1'-5\frac{1}{2}$ " Varies 1'-6" Max. in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 9" Max \sim W4.4 b. Place Conventional Reinforcement Bars 6A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" ∠ D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, D-1, D-2 & D-3) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift Pieces K FF = Front Face PIECE D-2 PIECE D-3 PIECE D-1 PIECES D & Bars 5Y to accommodate skewed end conditions and align BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 84 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20084 2 of 2



ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS Bars 5Y (16 Required) -Wires D31 (shown $21 \sim D16's @ 6'' sp. = 10'-0''$ D16's @ 1'-0" sp. as(O) Typ.) End of Varies Varies 9" Max. Beam Optional W6.4 W6.4 1'-0" Max. - Optional W6.4 W6.4 D16 Optional W6.4 D16 ----------W6.4 21/4" Cover Pieces K-1 Pieces K-2 1" extension (Offset) (Typ.)SECTION A-A PLAN VIEW PLAN VIEW FOR WELDED WIRE REINFORCEMENT $5\frac{1}{2}$ " \pm -PIECE M-3 PIECES M PIECE M-1 Piece M-1 tied Match spacing of END VIEW (2 Required) (2 Required) adjacent Piece S-1, Pieces S (Single Mat) Tied to Piece K-2 S-2, S-3 or S-4 to Strands at @ Beam) 17 ~ D25's (FF) @ 6" = 8'-0" S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam -6 ~ D31's @ 31/3" 17 ~ D25's (BF) @ 6" = 8'-0" S2 ~ D25's @ 9" sp. (Piece S-2) $sp. = 1'-5\frac{1}{2}"$ S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 2" Cover 3" Offset 25/8" Cover (Typ.) Varies 9" Max. 3¾" ± (Тур.) (Typ.)PARTIAL SECTION AT CENTER BEAM Pieces K (Pairs) ~W12.4 (Piece K-1) Pieces M-1 W10 (Pieces K-2 & S) W10) → D31 (Piece K-1) D25 (Pieces K-2 & S) 2" Cover Wires (W12.4 or 21/4" Cover -W12.4 (Piece K−1) ___Q Beam (WWR Ø Symmetrical) W10 (Pieces K-2 & S) Pieces D Piece Cover (Pairs) Cross 1" extension (Typ.) PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 END VIEW (Aligned EF) (FF Shown Solid, BF Shown Dashed) (2 Required ~ Each Piece) PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, C, Y (4 Required ~ (4 Required) Piece D-1 ties and Strands N not Shown for Clarity) 2 Pairs) to Piece K-1 — NOTES: $6 \sim D11's @ 3\frac{1}{2}"$ 1'-3" 17 ~ D11's @ 6" sp. = 8'-0" D11's @ 1'-6" spaces a. See Sheet 1 for placement details & Table of Beam Variables $sp. = 1'-5\frac{1}{2}"$ Varies 1'-6" Max. in Structures Plans for variables S1, S2, S3, S4 & V1. Varies 9" Max -W4.4 b. Place Conventional Reinforcement Bars 6A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & 31/4" ∠ D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, D-1, D-2 & D-3) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index No. 20010 EF = Each FaceSkew Details and Note 9 for placement details. Shift Pieces K FF = Front Face PIECE D-2 PIECE D-3 PIECE D-1 PIECES D & Bars 5Y to accommodate skewed end conditions and align BF = Back Face(4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) END VIEW with Bars C and D. DESCRIPTION: INDEX SHEET FY 2016-17 **REVISION** NO. NO. FDOT FLORIDA-I 96 BEAM - STANDARD DETAILS DESIGN STANDARDS 07/01/10 20096 2 of 2

BEAM NOTES

- 1. All bar dimensions are out-to-out.
- 2. Place one (1) Bar 4K, or 5Z at each location as detailed alternating the direction of the ends for each bar (see "ELEVATION AT END OF BEAM", Sheet 3).
- 3. Strands N shall be ASTM A416, Grade 270, seven-wire strands 🐉 Ø or larger, stressed to 10,000 lbs. each.
- 4. For beams with ends not to be encased in permanent concrete diaphragms, after detensioning cut wedge to recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2.
- 5. For beams with ends not to be encased in permanent concrete diaphragms, protect end of recessed strands in accordance with Specification Section 450.
- 6. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 7. At the Contractor's option, welded deformed wire reinforcement may be used in lieu of Bars 3D, 4K, and 5Z as shown on Sheet 4. Welded deformed wire reinforcement shall meet requirements of Specification Section 931.
- 8. Safety Line Anchorage Devices or sleeves are required and permitted in the top flange only to accommodate fall protection systems used during construction. See shop drawings for details and spacing of any required embedments.
- 9. For beams with skewed end conditions, the end reinforcement, defined as Bars 3D1, 3D2, 4K, 4Y and 5Z placed within the limits of Bars 3D in "ELEVATION AT END OF BEAM", shall be placed parallel to the skewed end of the beam. Bars 3D and 4K, located beyond the limits of Bars 3D shall be placed perpendicular to the longitudinal axis of the beam. For placement locations, see "SKEWED BEAM END DETAILS". Adjust the dimensions of Bars 3D1 and 3D2, as shown on the "BENDING DIAGRAM" for skewed end conditions.
- 10. Placement of Bars 3D1 correspond to END 1, and Bars 3C2, correspond to END 2. END 1 and END 2 are shown on the beam "ELEVATION".
- 11. For Beams with vertically beveled end conditions, place first row of Bars 3D1, 3D2, 4K, 4Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For welded deformed wire reinforcement, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
- 12. For beams with skewed end conditions, welded deformed wire reinforcement shall not be used for end confinement reinforcement (Bars 3D1 and 3D2).
- 13. Bars 4K and 5Z shall be placed and tied to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables in Structures Plans). For welded deformed wire reinforcement, supplemental transverse bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands or Strands N.
- 14. At the Contractor's option, Bars 3D1, 3D2 and 3D3 may be fabricated as a two-piece bar with a 1'-0" minimum lap splice of the bottom legs.
- 15. For referenced Dimensions, Angles and Case Numbers, see the Table of Beam Variables in Structures Plans.

DETAILS AND NOTES

REVISION 07/01/14

DESCRIPTION:

FDOT

FY 2016-17 DESIGN STANDARDS

(Showing Vertical Bevel of Beam End)

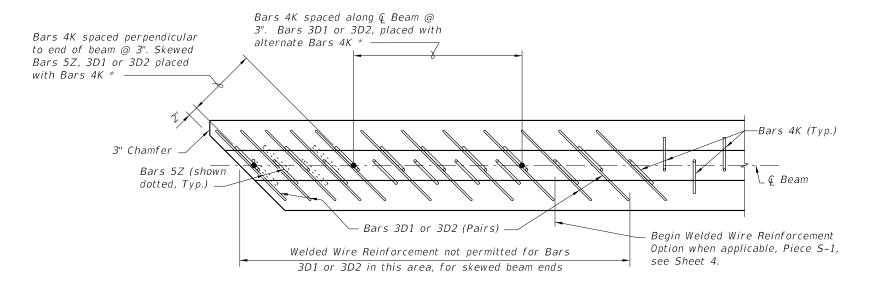
INDEX NO. 20120

NO. 1 of 4

SHEET

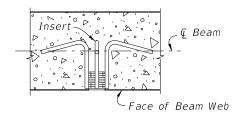
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for Welded Wire Reinforcement.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE) (End 1 Shown, End 2 Similar) (Bars 4Y & Strands not shown for clarity)

= SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



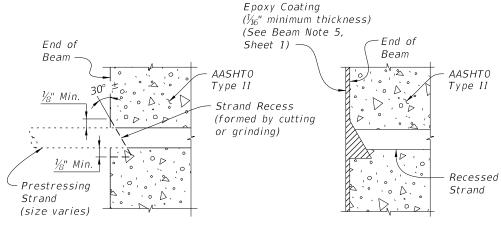
PLAN SECTION THRU BEAM WEB AT INSERT FOR DIAPHRAGM REINFORCING

(When Intermediate Diaphragms are Required by Design)

INSERT NOTES

- 1. Provide 1" Ø, zinc-electroplated, ferrule wing nut or coil inserts, UNC threads, 1/0 minimum gage wire, not more than 4" in depth with a minimum ultimate tensile strength of 11,400 lbs. in 4,000 psi concrete.
- 2. If inserts are needed on both sides (faces) of beam webs, an assembly as long as the thickness of the beam web, consisting of two (2) ferrule or coil inserts attached by two (2) or more struts may be utilized. The connecting struts shall have a minimum ultimate tensile strength of 11,400 lbs.
- 3. Inserts for diaphragm reinforcing are required at each end of each intermediate diaphragm shown on the Beam Framing Plan and may be required at the end of the beams when end diaphragms are shown. See Superstructure and Beam Framing Plans for longitudinal location of inserts for each face of beam.

==== INSERT DETAIL ===



TYPICAL SECTION SHOWING CUT STRAND RECESS LIMITS

TYPICAL SECTION AFTER PROTECTING

=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

REVISION 07/01/14

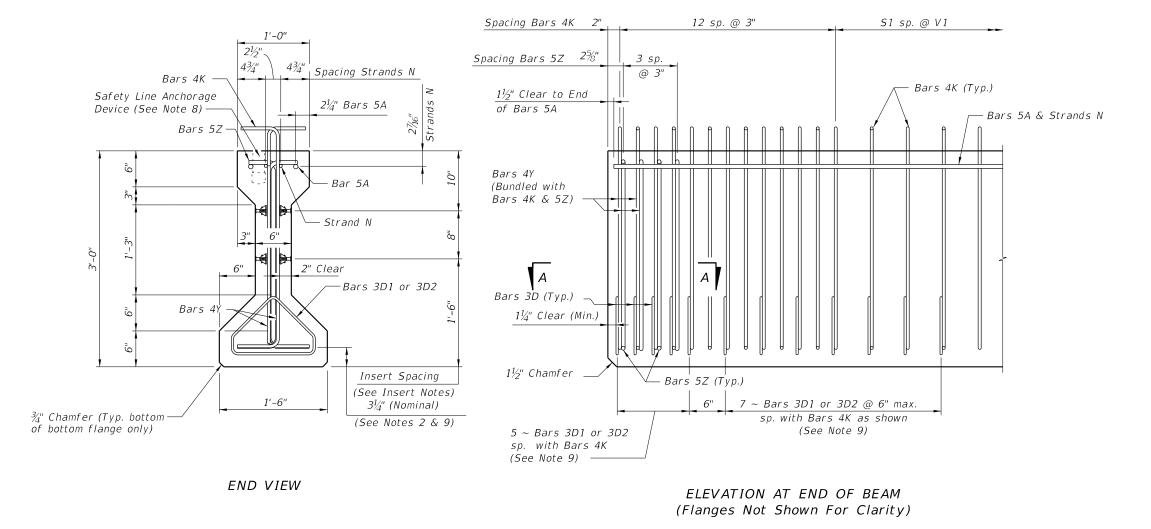
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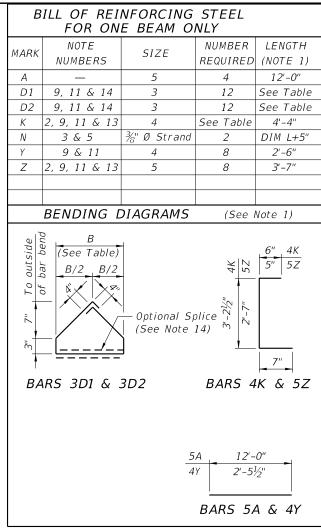
FY 2016-17 **DESIGN STANDARDS**

AASHTO TYPE II BEAM

INDEX NO. 20120

SHEET NO. 2 of 4



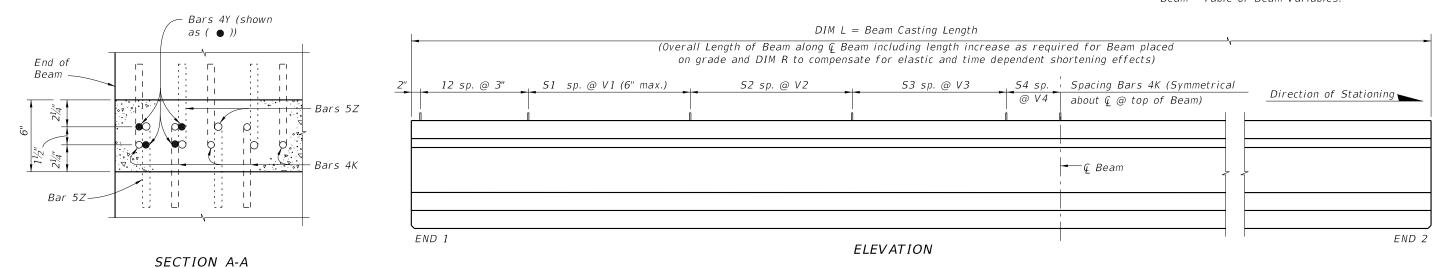


NOTES

Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.



12/17/2015 1:

LAST ODESCRIPTION:
REVISION OT/01/13

(Showing Bars 4K, 4Y & 5Z Only)

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FY 2016-17 DESIGN STANDARDS

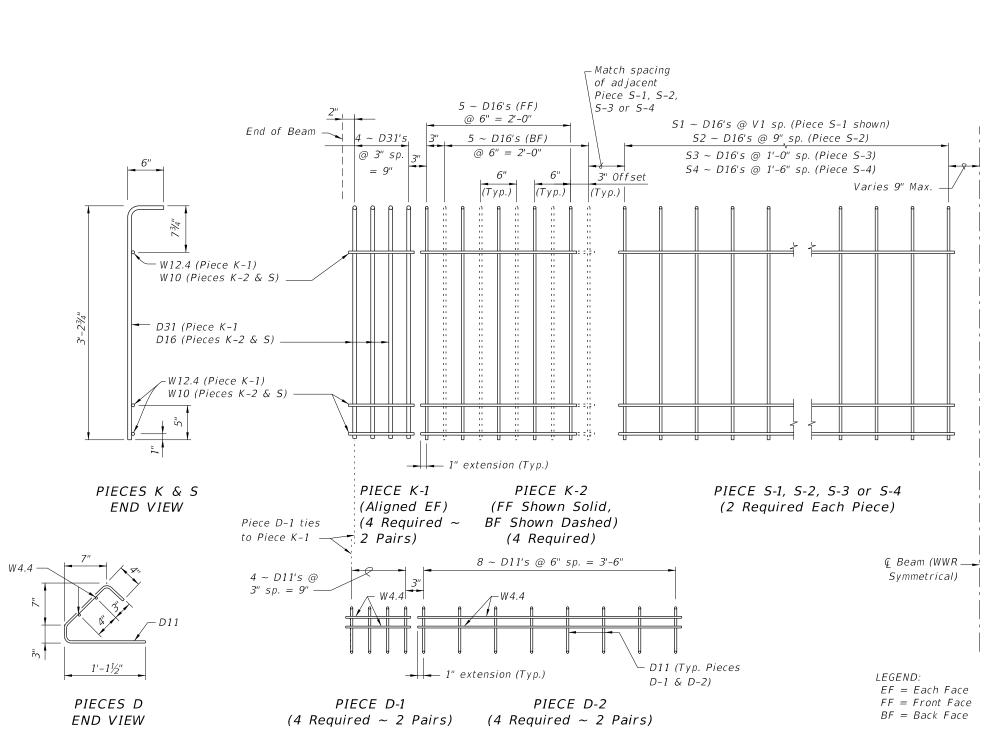
AASHTO TYPE II BEAM

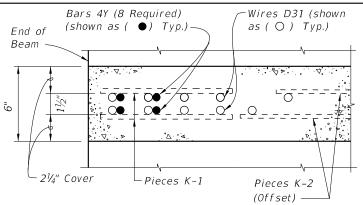
INDEX NO. 20120

STANDARD DETAILS

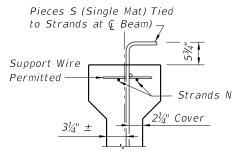
SHEET NO. **3 of 4**

ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS

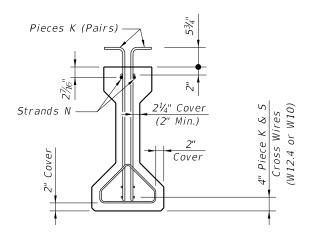




SECTION A-A FOR WELDED WIRE REINFORCEMENT



PARTIAL SECTION AT CENTER BEAM



PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, 54 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for Welded Wire Reinforcement. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

STANDARD DETAILS

REVISION 07/01/13

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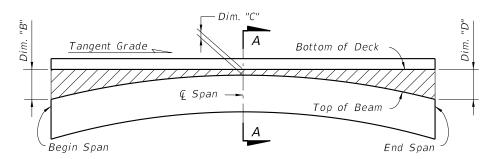
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FY 2016-17 **DESIGN STANDARDS**

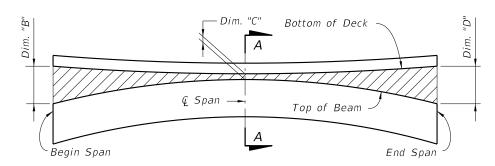
AASHTO TYPE II BEAM

INDEX NO. 20120

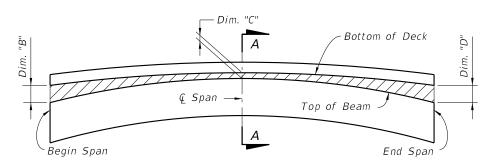
SHEET NO. 4 of 4



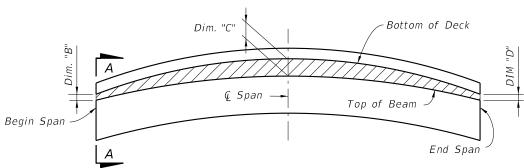
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG Q BEAM) (CASE 3)

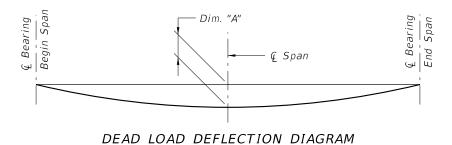


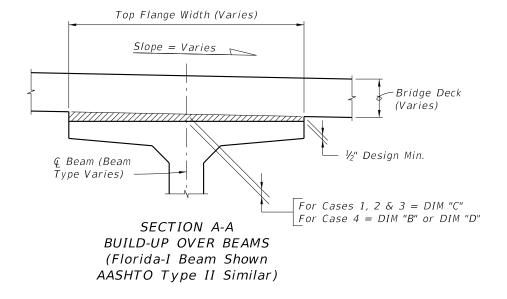
BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG Q BEAM) (CASE 4)

BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

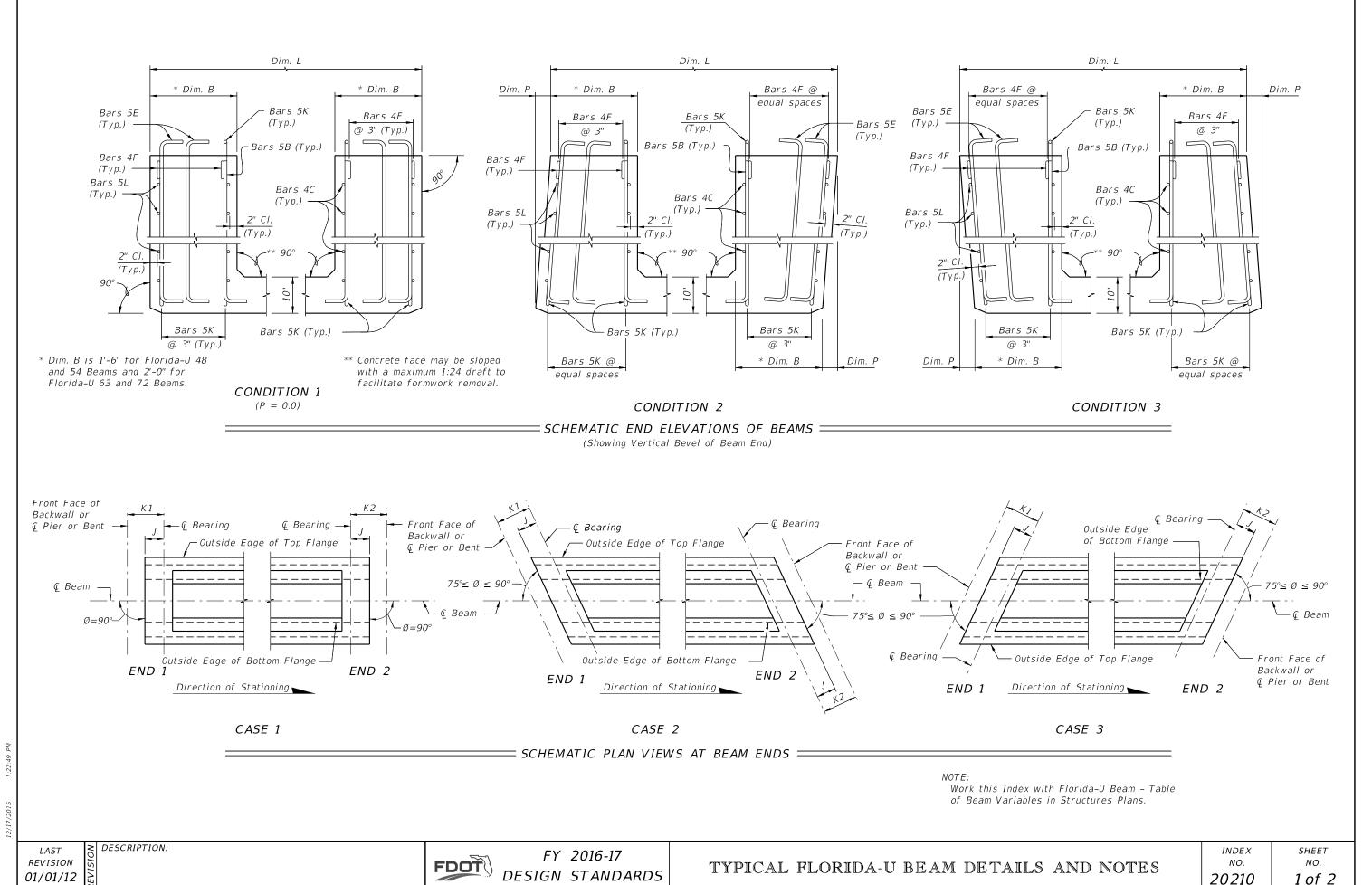
Dim. "A" includes the weight of the Stay-In-Place Formwork.

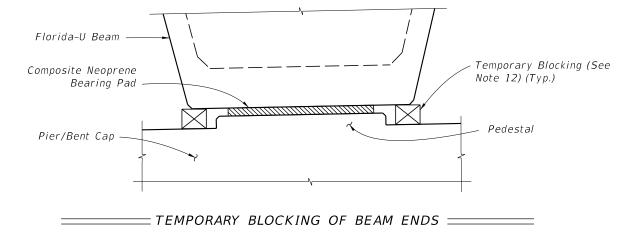


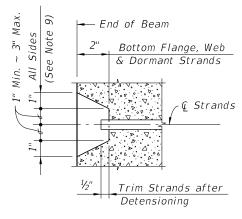


* NOTE:

Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.







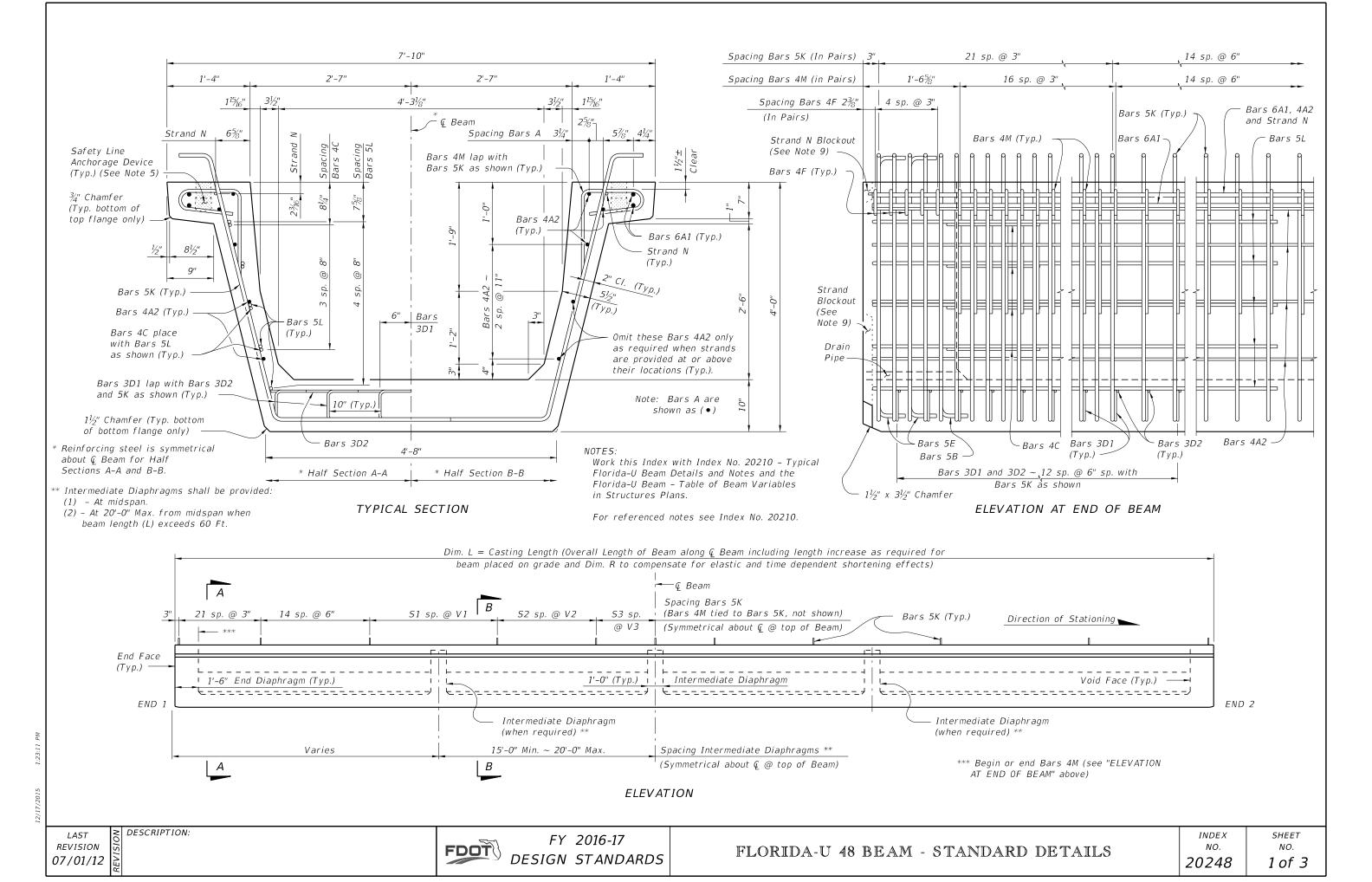
TYPICAL STRAND BLOCKOUT DETAIL

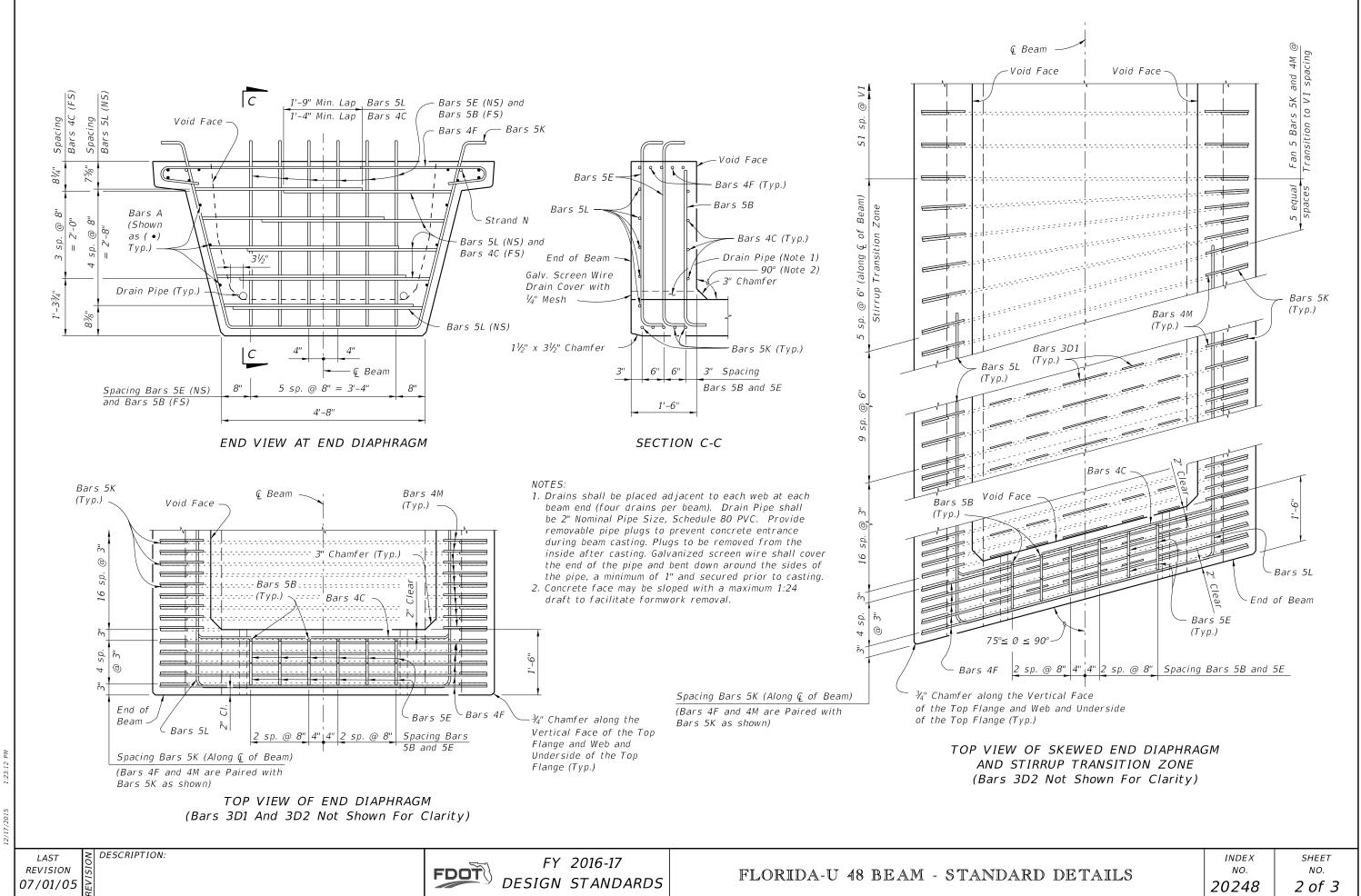
BEAM NOTES

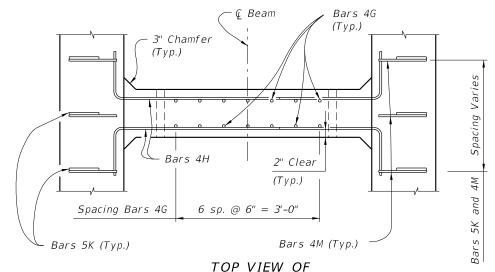
- 1. All bar dimensions are out-to-out.
- 2. Strands N (Dormant Strands) shall be ASTM A416, Grade 270, seven-wire strands 3/8" Ø or larger, stressed to 10,000 lbs. each.
- 3. Unless otherwise noted in Structures Plans, the minimum concrete cover for reinforcing steel shall be 2".
- 4. At the option of the Contractor and with the Engineer's Approval, deformed welded wire reinforcement (WWR) may be used in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M except as noted below in note 7, provided the wire sizes and spacing match those shown on the Standard Beam Detail sheets for these bars. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.
- 5. Safety Line Anchorage Devices or sleeves are required and permitted in the top flanges only to accommodate fall protection systems used during construction. See shop drawings for details and spacing of any required embedments.
- 6. For Beams with vertically beveled end conditions when "Dim. P" exceeds 1", Bars 5E and the first Bars 4F and 5K shall be placed parallel to the end of the beam. The remaining Bars 4F and 5K within the limits of "Dim. B" shall be fanned at equal spaces.
- 7. Welded deformed wire reinforcement shall not be used for the end reinforcement (Bars 5B, 4C, 3D, 5E, 4F, 5K, and 5L) for beams with skewed end conditions or vertically beveled end conditions when "Dim. P" exceeds 1".
- 8. Bars 5K shall be placed and tied to the fully bonded strands in the bottom row (see "STRAND PATTERN" in Structures Plans).
- 9. Strand Protection at beam ends shall consist of a 2" deep recess formed around all strands (including dormant) or strand groups. Extend recess to face of web and bottom of flange for bottom row of strands. After detensioning, cut strands 1/2" from recessed surface and fill the recess with a Type F-2 or Q Epoxy Compound in accordance with Section 926 of the Specifications.
- 10. Use Size No. 67 maximum sized aggregate.
- 11. Use Stay-in-Place metal deck forms inside the beams.
- 12. Prior to deck placement, based on the deck forming system and deck placement sequence, evaluate and provide, if necessary, temporary bracing between the U Beams. Also, prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations that occur during placement of the deck. Leave temporary blocking and bracing in place for a minimum of four days after the deck placement.
- 13. For referenced Dimensions, Angles and Case Numbers see Table of Beam Variables in Structures Plans.

Work this Index with Florida-U Beam - Table of Beam Variables in Structures Plans.

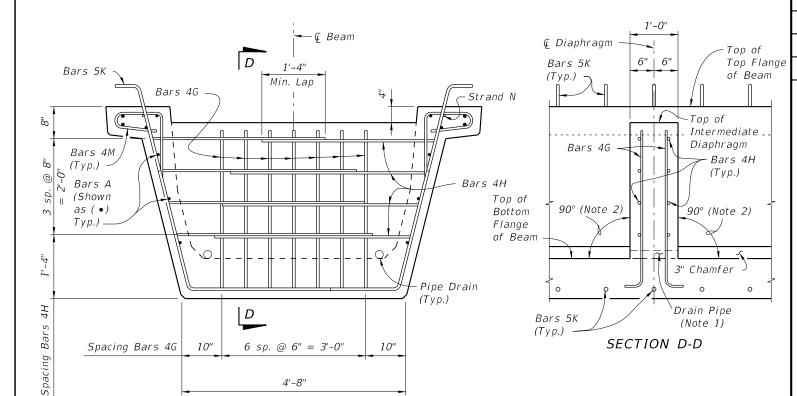
DESCRIPTION:







INTERMEDIATE DIAPHRAGM

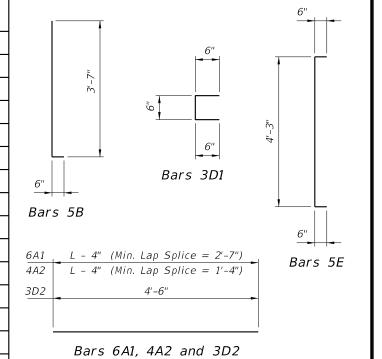


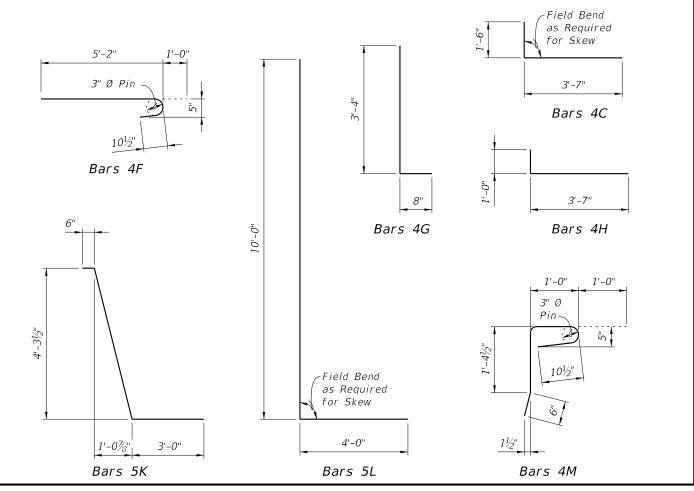
SECTION AT INTERMEDIATE DIAPHRAGM

- 1. Drains shall be placed adjacent to each web at each intermediate diaphragm (two drains per intermediate diaphragm). Drain Pipe shall be 2" Nominal Pipe Size, Schedule 80 PVC. Provide removable pipe plugs to prevent concrete entrance during beam casting. Plugs to be removed from the inside after casting.
- 2. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 3. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing beam from casting bed.

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL





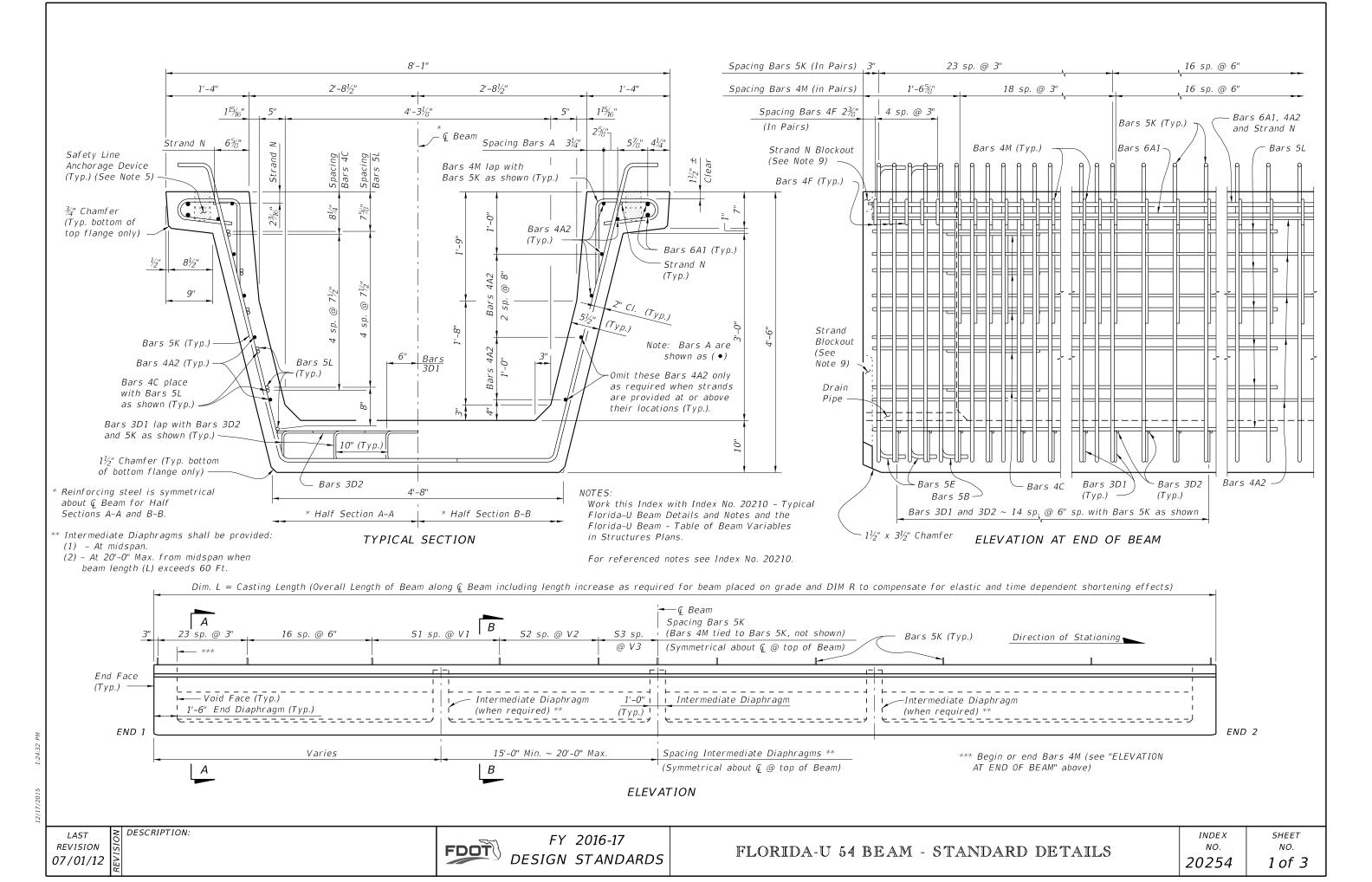


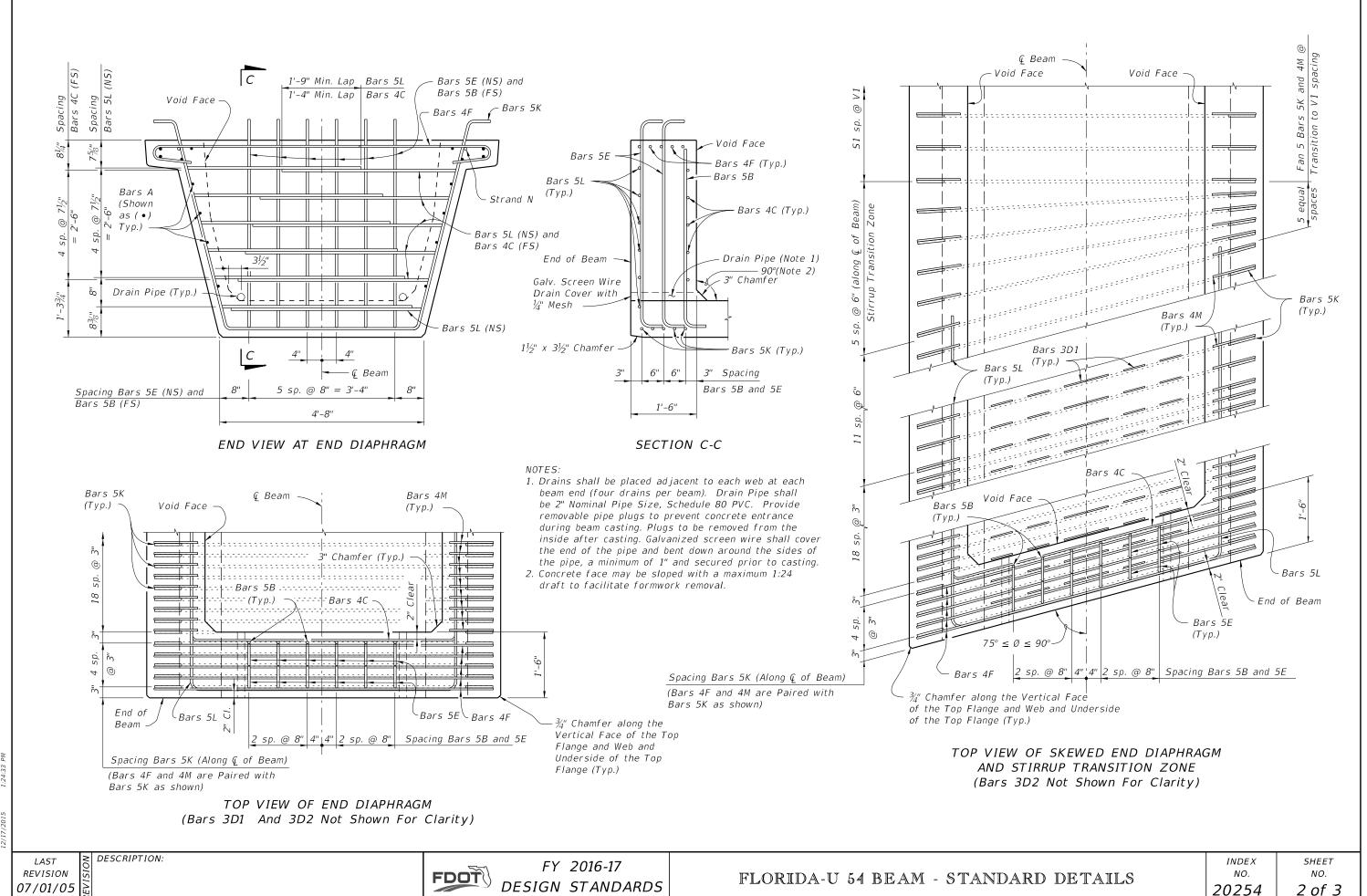
REVISION 01/01/11

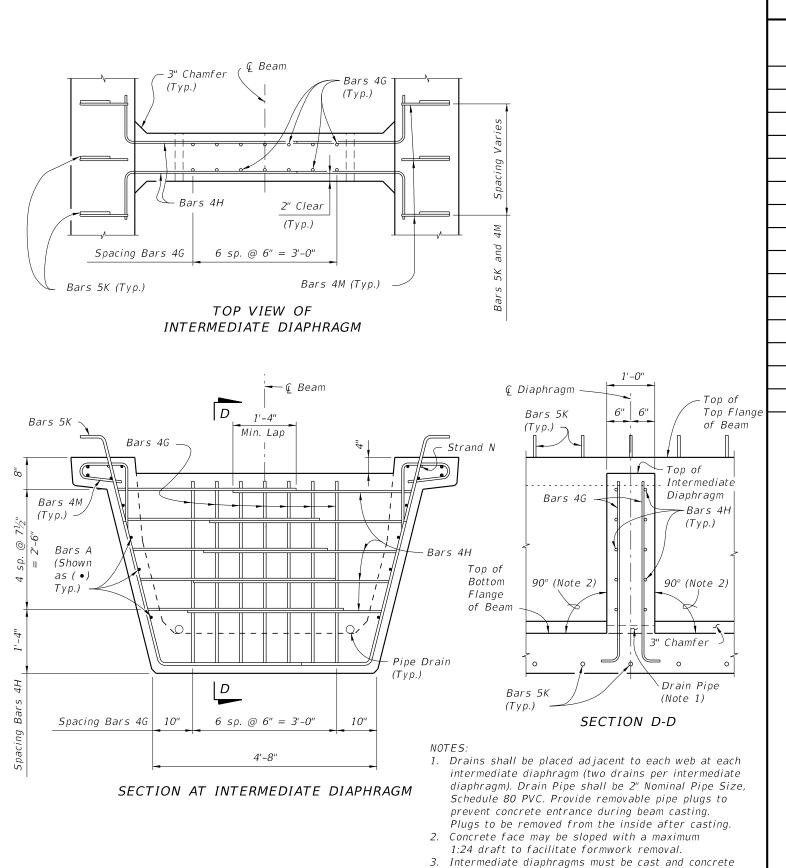
DESCRIPTION:

FDOT

FY 2016-17 **DESIGN STANDARDS**







CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL FOR ONE BEAM ONLY MARK SIZE NO. REQD. LENGTH A1 6 Dim. L - 4" 4 A2 12 Dim. L - 4" В 5 12 4'-7" С 4 20 5'-3" D1 3 180 1'-6" Bars 3D1 D2 3 30 4'-6" 6" Ε 5 24 5'-9" Bars 5B 4 20 6'-4" G 4 See Table 4'-6" L - 4" (Min. Lap Splice = 2'-7") Bars 5E L - 4" (Min. Lap Splice = 1'-4") Н 4 See Table 4'-9" 3D2 4'-6" 5 See Table 8'-6" 5 24 16'-2" L Μ 4 See Table 3'-11" Bars 6A1, 4A2 and 3D2 Ν ¾" Ø Strand 2 Dim. L - 3" Field Bend as Required for Skew 5'-4" 3" Ø Pin 3'-9" Bars 4C Bars 4F 3'-9" Bars 4G Bars 4H 1'-0" 1'-0" 3" Ø Pin-Field Bend as Required for Skew 4'-6" 1'-23/8" 3'-0" Bars 5K Bars 5L Bars 4M

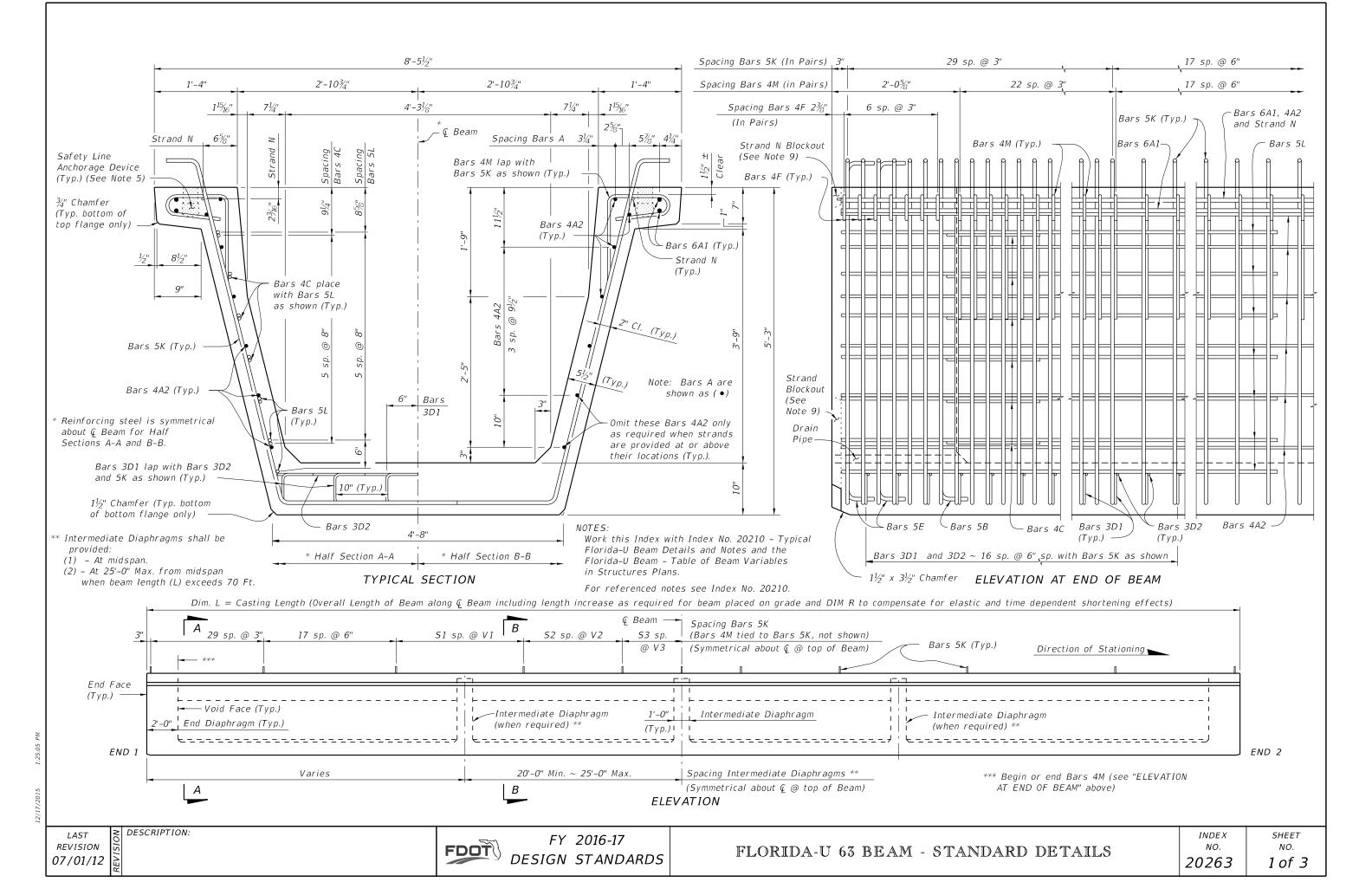
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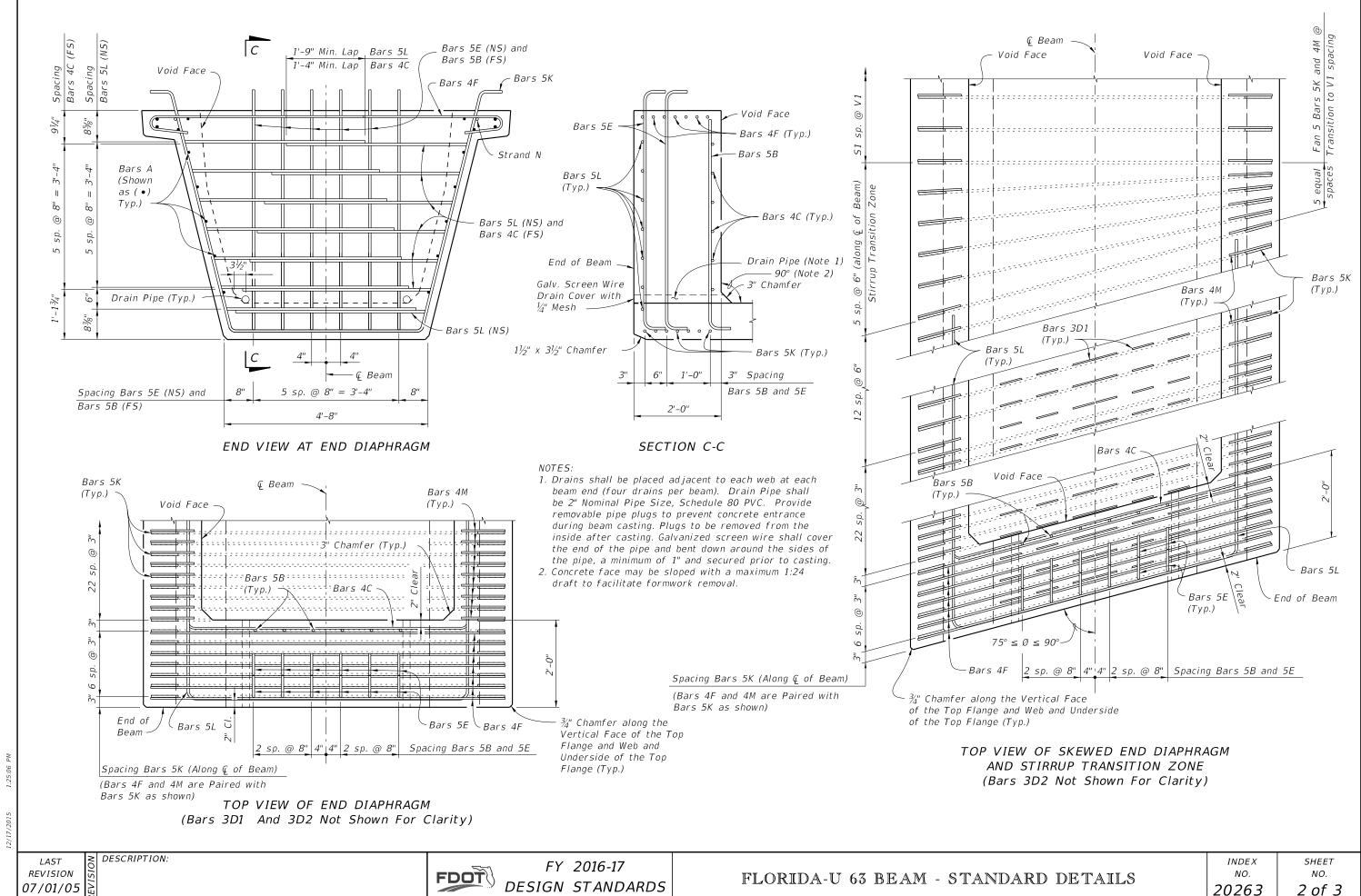
01/01/11

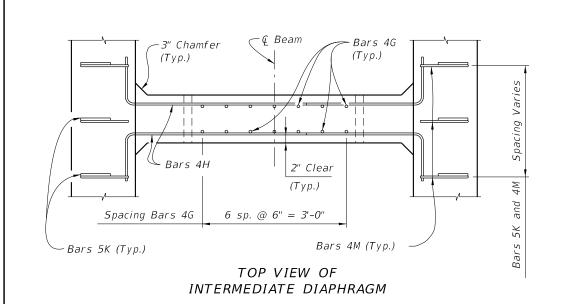
from casting bed.

FY 2016-17 **DESIGN STANDARDS**

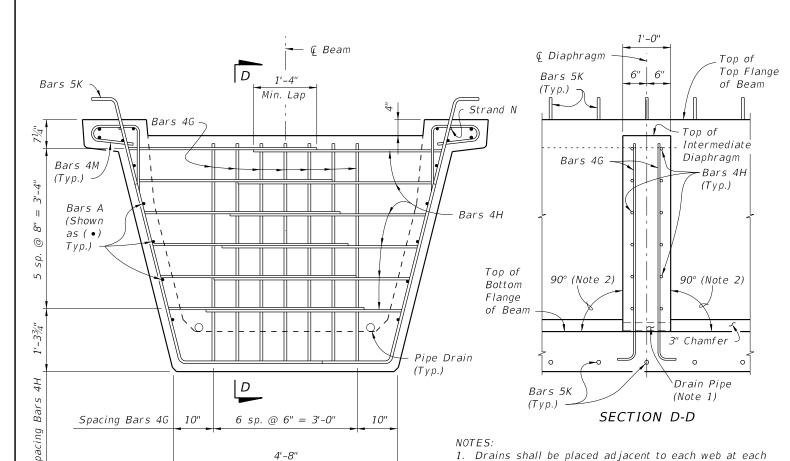
release strength obtained prior to removing beam







SECTION AT INTERMEDIATE DIAPHRAGM



MARK SIZE NO. REQD. LENGTH A1 6 4 Dim. L - 4" A2 4 12 Dim. L - 4" 12 В 5 5'-4" С 4 24 5'-5" D1 3 204 1'-6" Bars 3D1 D2 3 34 4'-6" 6" Ε 5 24 6'-6" Bars 5B 4 28 6'-6" G 4 See Table 5'-3" L - 4" (Min. Lap Splice = 2'-7") Bars 5E L - 4" (Min. Lap Splice = 1'-4") 4 4'-11" Н See Table 3D2 4'-6" Κ 5 9'-21/5" See Table 5 28 17'-8" L Μ 4 See Table 3'-11" Bars 6A1, 4A2 and 3D2 ¾" Ø Strand Dim. L - 3" -Field Bend as Required for Skew 5'-6" 3" Ø Pin 3'-11" Bars 4C Bars 4F 3'-11" 8"_ Bars 4G Bars 4H 1'-0'' 1'-0" -6½" -Field Bend as Required for Skew 4'-6" 1'-45/6" 3'-0" Bars 5K Bars 5L Bars 4M

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL

FOR ONE BEAM ONLY

intermediate diaphragm (two drains per intermediate diaphragm). Drain Pipe shall be 2" Nominal Pipe Size,

Schedule 80 PVC. Provide removable pipe plugs to

Plugs to be removed from the inside after casting.

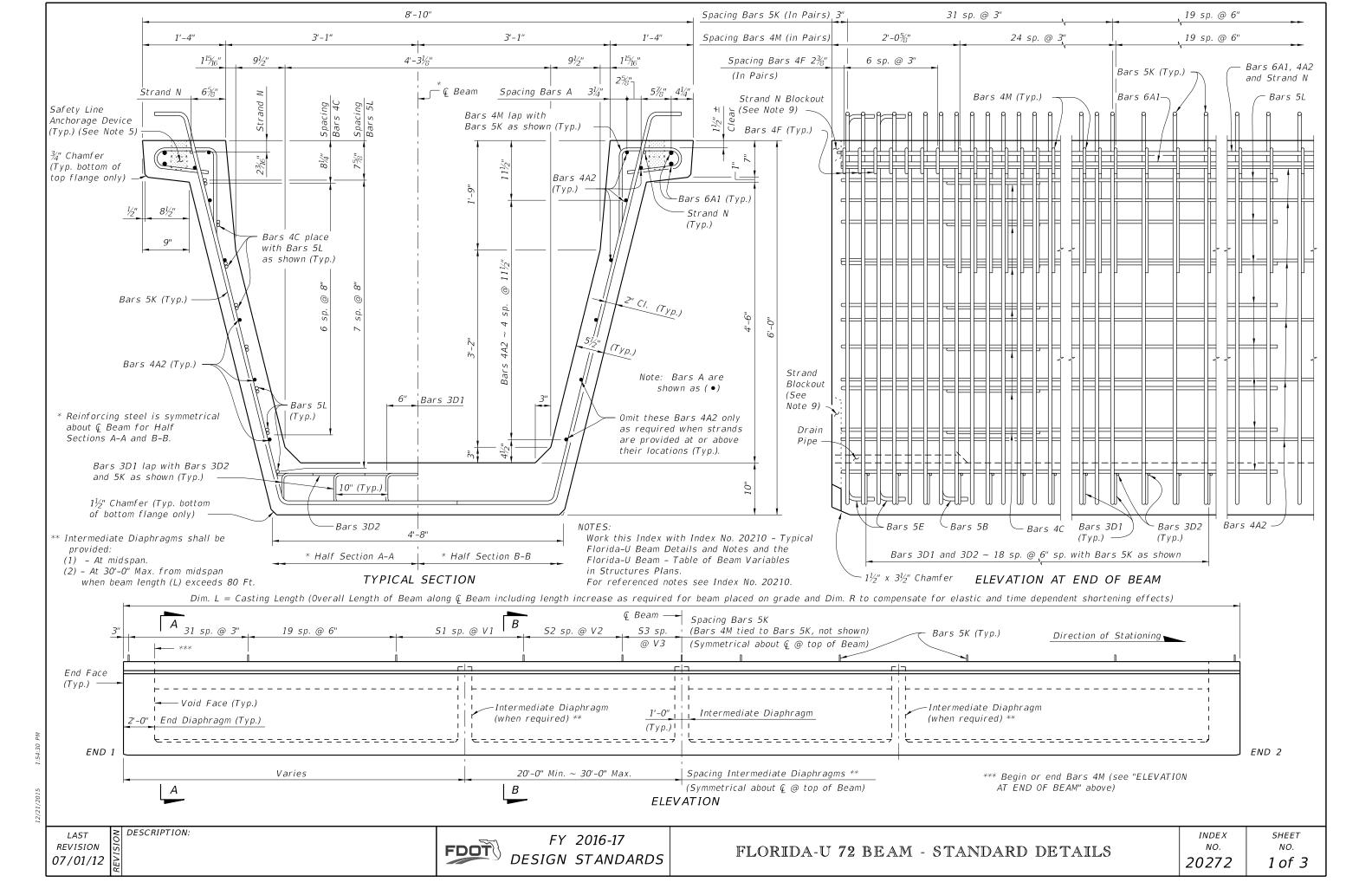
prevent concrete entrance during beam casting.

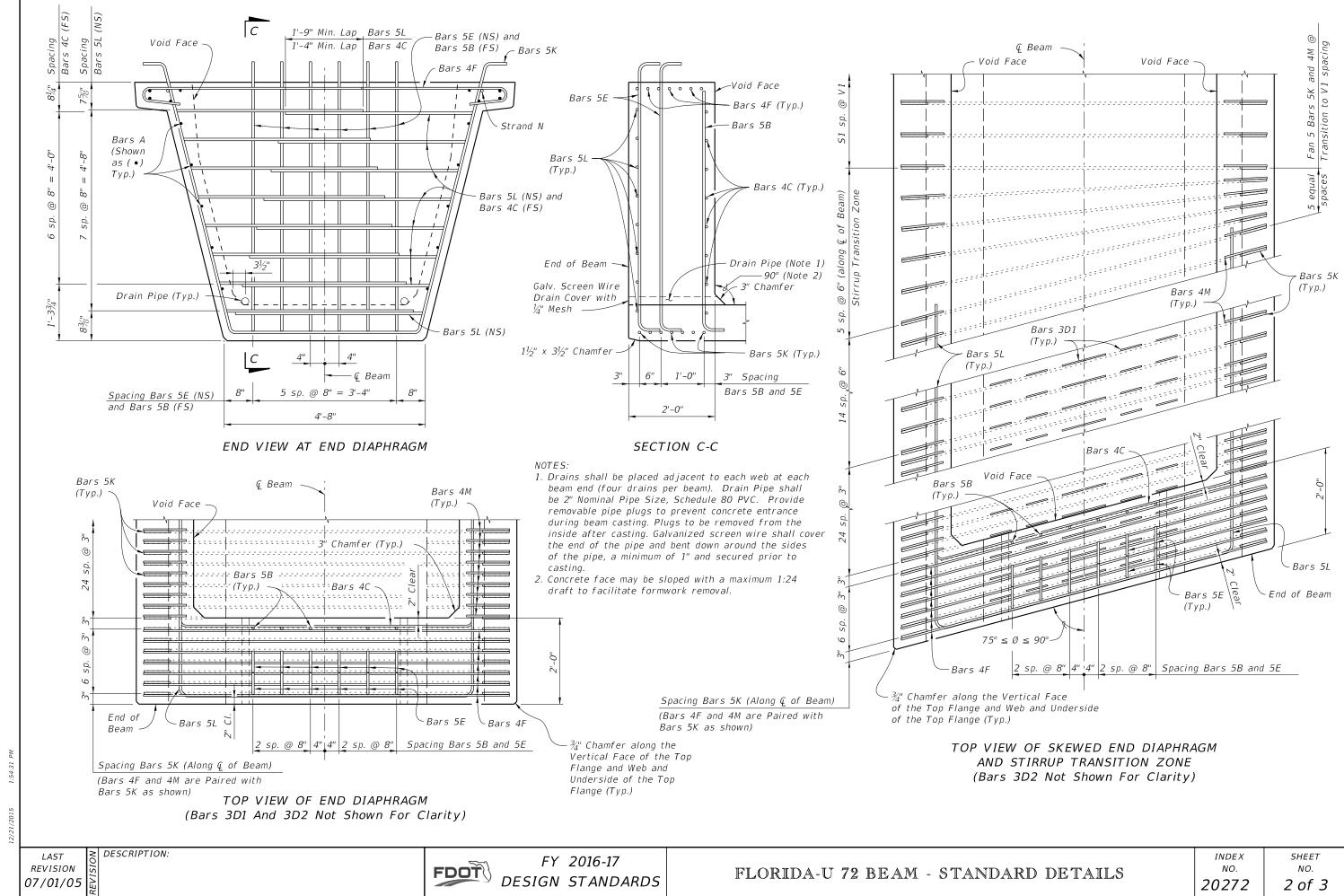
3. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing beam

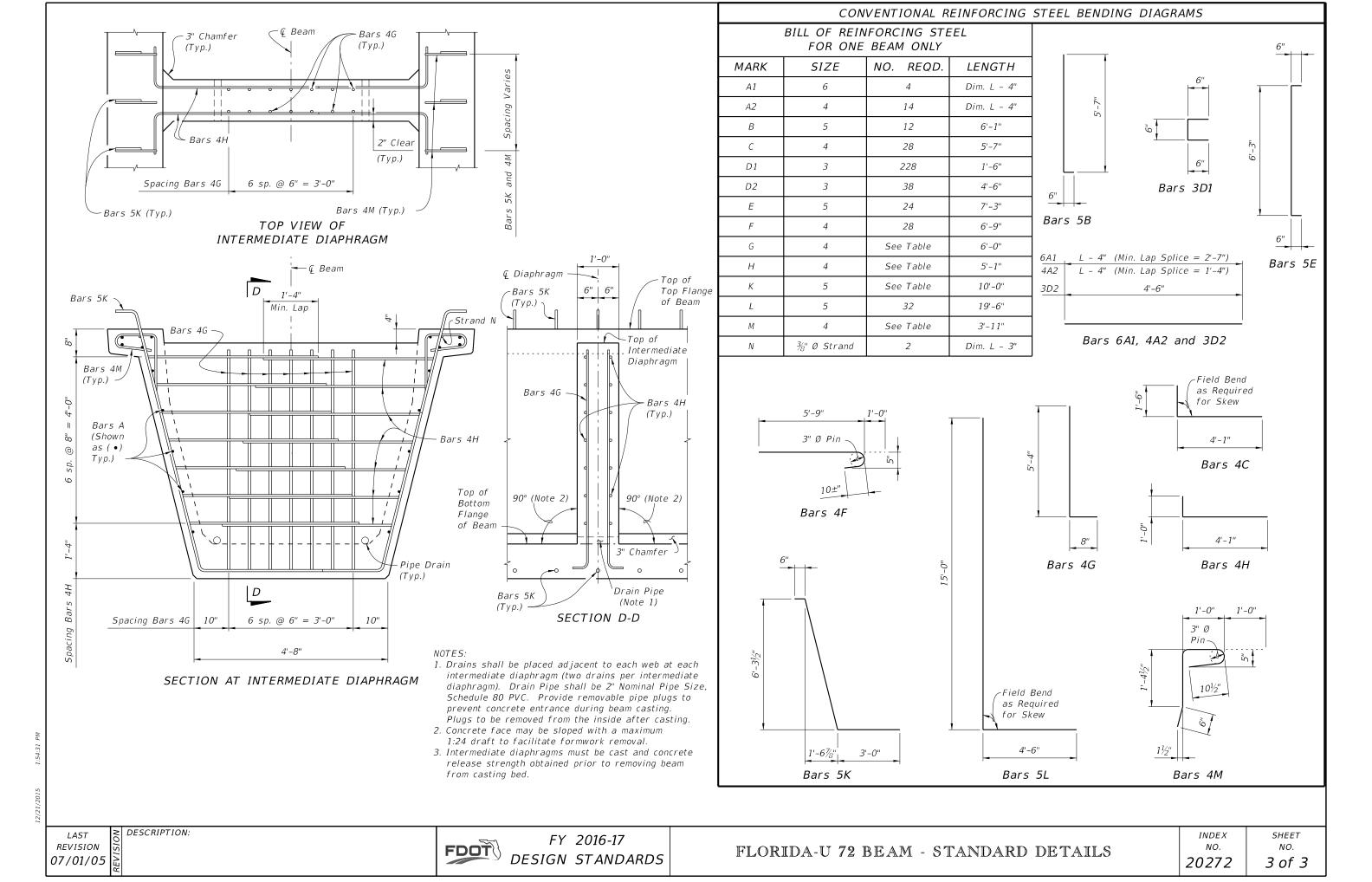
2. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.

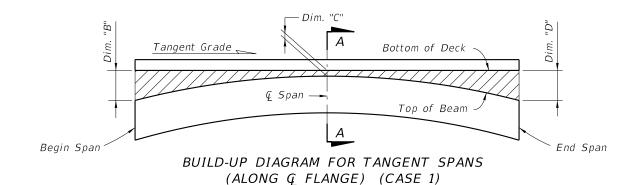
from casting bed.

SHEET



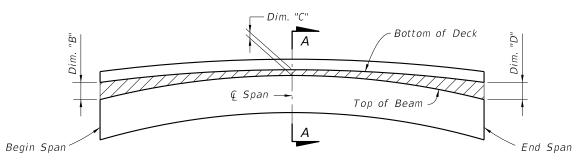




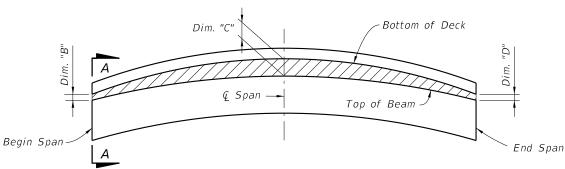


Begin Span Dim. "C" Bottom of Deck A End Span End Span

BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS
(ALONG Q FLANGE) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT Q SPAN
(ALONG Q FLANGE) (CASE 3)

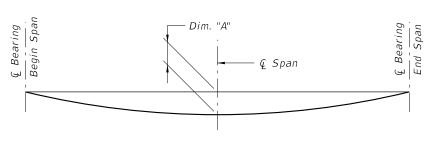


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT BEGIN OR END SPAN
(ALONG Q FLANGE) (CASE 4)

BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than $+/-\frac{1}{2}$ " from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM
(ALONG Q BEAM)

