### GENERAL NOTES

- 1. The illustrations for quardrail applications are standard configurations; adjustments are to be made as required by site specific conditions to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figures 1 and 2, and other application details of this Index.
- 3. One Panel (i.e., panel length) equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standard indexes or where specifically called for in the plans.

Post spacing shall be 6'-3" except that reduced spacing shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Detail J and Index No. 402) and transitions to redirective crash cushions. (b) the conditions in Note No. 7 below. (c) special post applications, (d) reduced post spacing required for specific end anchorage assemblies, and, (e) specific spacing called for in the plans.

- 4. The standard quardrail mounting height for W-beam quardrail is 2'-1" and for thrie-beam quardrail is 1'-9" to the center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a deviation of 1" below and 3" above the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines). For guardrail placed on slopes beyond the shoulder point, there shall be no deviation more than 1" below to 3" above the desired height within any 25 foot section of guardrail. For standard guardrail with a mounting height of 2'-1" to the center of beam, a construction tolerance of  $\frac{1}{2}$ " below and 1" above the standard mounting height is permissible. Use the applicable 2013 Design Standards, Index 400 Series for repair or replacement of existing W-beam guardrail systems with a mounting height of 1'-9" to the center of beam.
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end anchorage for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end anchorages will be constructed only when restraints prevent construction of flared end anchors.

Guardrail end anchorage assemblies shall be of the type called for in the plans. If the plans call for a "flared" end anchorage assembly and does not identify the specific system to be used, the contractor has the option to construct any FDOT approved "flared" end anchorage assembly identified on the Approved Products List (APL), subject to the conditions identified in these drawings, or the approved APL drawings.

If the plans call for a "parallel" end anchorage assembly and does not identify the specific system to be used, the contractor has the option to construct any FDOT approved "parallel" end anchorage assembly identified on the APL, subject to the conditions identified in these drawings, or the approved APL drawings.

If the plans call for a specific end anchorage assembly, substitutions with other end anchorage assemblies will not be permitted unless approved by the Engineer. Approved substitutions will not be eligible for CSIP consideration.

When an end anchorage assembly is attached to guardrail and PEDESTRIAN SAFETY TREATMENTS are required, only end anchorage assemblies approved with timber posts are to be used.

Currently approved proprietary end anchorage assemblies are identified on the Approved Products List (APL). Manufacturers seeking approval of proprietary end anchorage assemblies for inclusion on the APL must submit an application with appropriate documentation showing that the end anchorage assembly is deemed eligible by the Federal Highway Administration (FHWA) for federal funding on the National Highway System (NHS) and is compatible with FDOT guardrail systems. System approvals will be contingent upon FDOT's evaluation of crash test performance results for consistency with FDOT system applications and use. If approved, product drawings signed and sealed by a professional engineer licensed in the State of Florida is required.

- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 5' minimum for standard W-beam, other guardrail configurations with reduced post spacing may be applicable; see General Note No. 11 and the minimum offset table on Sheet 19. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier wall shall be used unless other shielding is approved by the Engineer of Record. See Index No. 410 for safety shape concrete barrier walls and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at roadside hazards or other areas where the Engineer has deemed guardrail necessary, guardrail should be considered on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone and fill heights are 6' or greater. Curbed sections should be evaluated for installation of guardrail where fill slopes are steeper than 1:3 and fill heights are 6' or greater within 22' of the traveled way. For additional details on curbed sections, see DETAIL L, LOCATION AT CURB & GUTTER SECTIONS
- 9. The guardrail to bridge connections contained in this Index are for bridges with Test Level 4 traffic railing barriers. For guardrail to concrete barrier wall connections see Index No. 410. For existing bridges receiving retrofit traffic railing barriers see Index No. 402
- DESCRIPTION: LAST REVISION 06/06/14

2016

DESIGN STANDARDS

- 10. The W-beam guardrail system in this index is the standard system to be used on the State Highway System where a Test Level 3 semi-rigid barrier is required.
- 11. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic railing barriers, to concrete and certain water filled safety shaped barriers, certain crash cushions and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 402, 410 and 414. The use of thrie-beam guardrail with standard offset blocks (Test Level 3 semi-rigid system) may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
  - a. W-beam deflection is marginal.
  - b. W-beam with rub rail considered functionally deficient,
  - c. Vehicle overriding W-beam is probable,
  - d. Drainage will be impeded or blocked by the use of concrete barrier wall (subject to deflection space requirements),
  - e. High frequency of repairs to W-beam,
  - f. Spandrel beam with low deflection needed around unrelocatable structure,

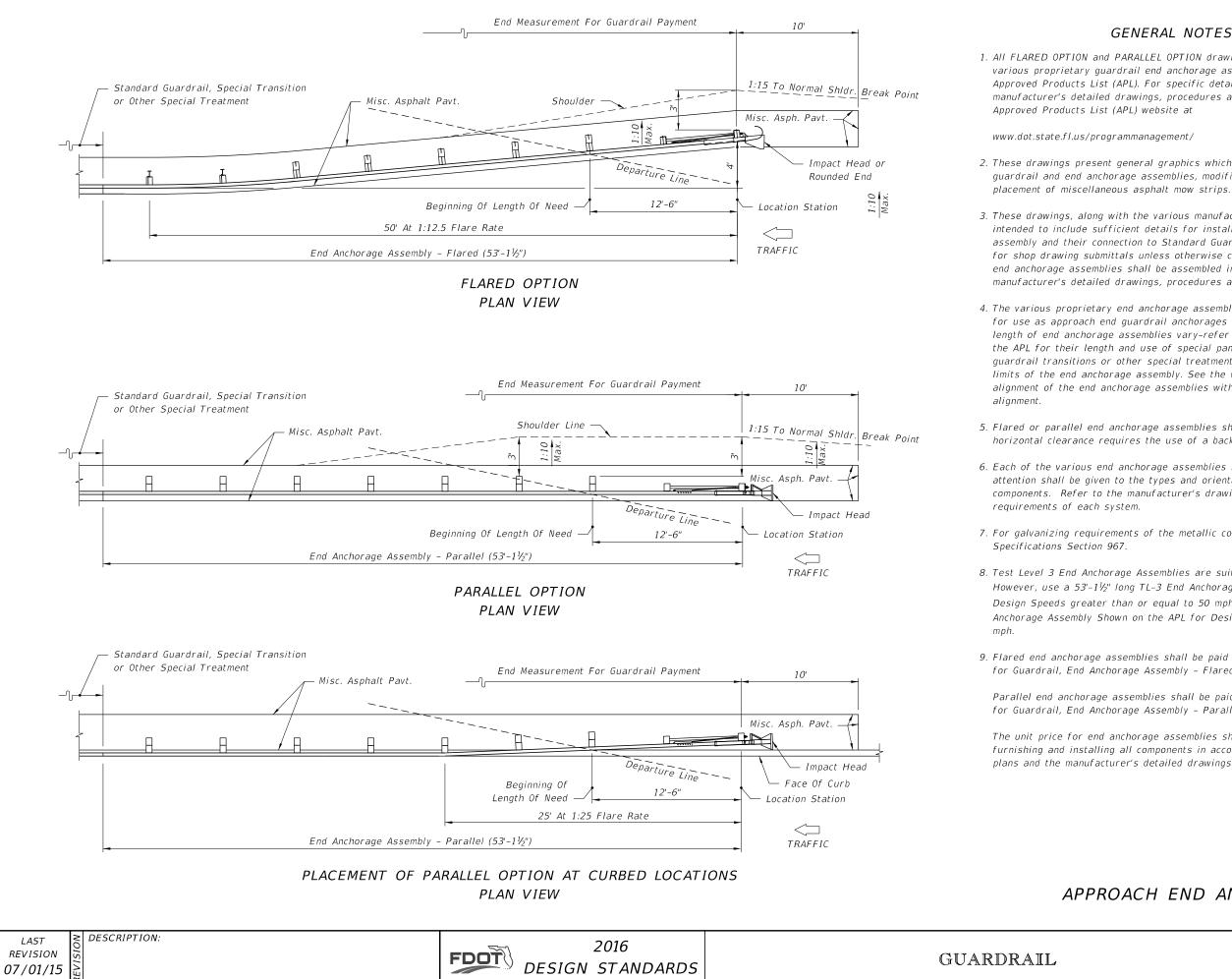
The modified thrie-beam guardrail is a Test Level 4 semi-rigid system and may be used where a Test Level 4 guardrail is required.

- 12. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
- a. Wide medians where approach end anchorage is located outside of opposing roadway clear zone,
- b. Medians of uniform width that are occupied by other transportation and joint use facilities,
- c. Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail installations,
- d. Medians of bifurcated roadways.
- 13. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 14. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crash worthiness at terminals. Crash cushions or Redirective Median End Anchorage Assemblies shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 15. Corrugated sheet steel beams, end shoes, end sections and backup plates shall conform to the requirements of AASHTO M180, Class A (12 Gauge), Type II (zinc) coating, except the W-Thrie Beam Transition Panel detailed on Sheet 20A shall be Class B (10 Gauge). All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTO requirements.
- 16. Offset blocks:
  - a. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing steel offset blocks may remain throughout the service life of the existing guardrail. PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS are tabulated on Sheet 16.
  - b. Composite offset block installations shall be constructed on guardrail outside of approach end anchorage assemblies or any transition system connecting to a rigid or thrie-beam barrier.
- 17. New holes in existing guardrail are to be punched. Where necessary to enlarge existing holes to guardrail, the work will be done by drilling or reaming. Repair damaged galvanization in accordance with Section 562. Burning of any holes will not be permitted.
- 18. For BARRIER DELINEATOR see DETAIL M.
- 19. Any run of guardrail with existing concrete posts that is being relocated under a construction or maintenance contract shall be replaced using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 20. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for CSIP consideration.
- 21. On roadways designated for reverse laning, all downstream ends of guardrail that are not shielded or that are not designed as approach end terminals shall be marked with post-mounted Type 3 Object Markers. Trailing bridge ends and trailing shoulder concrete barrier wall ends shall be marked with Type 3 Object Markers except where there is trailing end guardrail. Object markers to be installed facing reverse laning traffic. The cost of the object marker shall be included in the cost of the guardrail.

GUARDRAIL

g. Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g., passenger vans and small buses).

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1. All FLARED OPTION and PARALLEL OPTION drawings are representative of the various proprietary guardrail end anchorage assemblies listed on the Department's Approved Products List (APL). For specific details and requirements refer to the manufacturer's detailed drawings, procedures and specifications located on the

# GENERAL NOTES

2. These drawings present general graphics which depict the limits of payment for guardrail and end anchorage assemblies, modifications to the shoulder, and

3. These drawings, along with the various manufacturer drawings on the APL, are intended to include sufficient details for installation of the end anchorage assembly and their connection to Standard Guardrail. This precludes requirements for shop drawing submittals unless otherwise called for in the plans. The various end anchorage assemblies shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.

4. The various proprietary end anchorage assemblies listed on the APL are intended for use as approach end guardrail anchorages for Standard Guardrail. The actual length of end anchorage assemblies vary-refer to the manufacturer's drawings on the APL for their length and use of special panels and details. Standard guardrail, guardrail transitions or other special treatments shall not be included within the limits of the end anchorage assembly. See the manufacturer drawings for the alignment of the end anchorage assemblies with respect to the normal guardrail

5. Flared or parallel end anchorage assemblies shall not be used in medians where horizontal clearance requires the use of a back rail.

6. Each of the various end anchorage assemblies have unique features. Careful attention shall be given to the types and orientation of the posts and other components. Refer to the manufacturer's drawings on the APL for the specific

7. For galvanizing requirements of the metallic components see Standard

8. Test Level 3 End Anchorage Assemblies are suitable for all design speeds. However, use a  $53'-1\frac{1}{2}''$  long TL-3 End Anchorage Assembly shown on the APL for Design Speeds greater than or equal to 50 mph and a 40'-71/2" long TL-2 End Anchorage Assembly Shown on the APL for Design Speeds less than or equal to 45

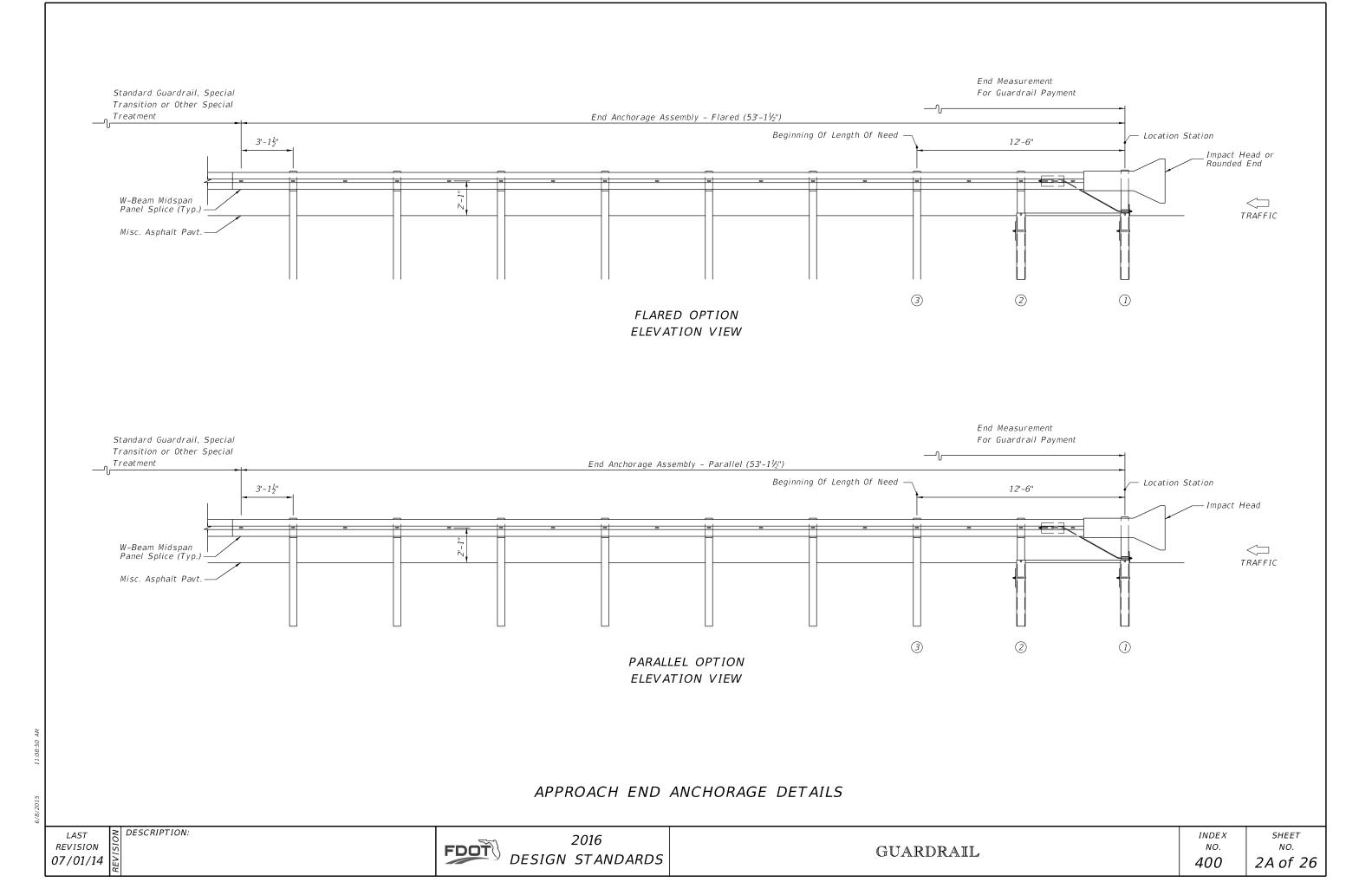
9. Flared end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly - Flared, EA.

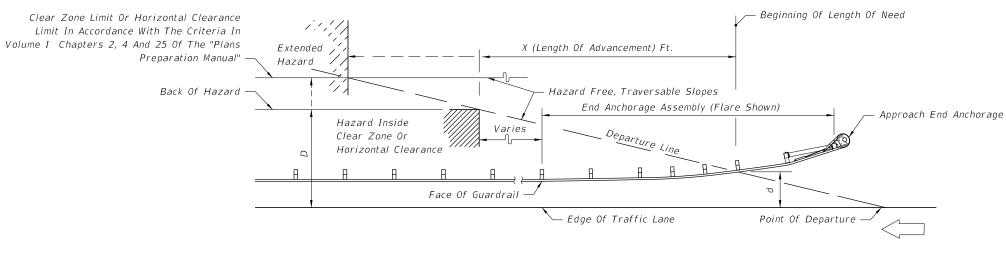
Parallel end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly - Parallel, EA.

The unit price for end anchorage assemblies shall be full compensation for furnishing and installing all components in accordance with these drawings, the plans and the manufacturer's detailed drawings, procedures and specifications.

# APPROACH END ANCHORAGE DETAILS

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Design Speed mph	X (Length Of Advancement) Ft. (See NOTES 1 & 2)
≤ 45	= 16 (D-d)
≥ 50	= 13 (D-d)

### NOTES

- 1. Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.
- 2. The flared end anchorage with 4' nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

### Equation Variables:

D = Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

d = Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

For flared and parallel end anchorage assemblies the beginning length of need is to be set at the center of post #3. That is, the departure line must intersect the face of the rail at post #3.

For flared end anchorage assemblies the offset distance "d" will equal the normal guardrail offset measured from the face of the guardrail to the edge of the near approach travel lane plus 1'-2" for 45 mph or less and 1'-91/4" for greater than 45 mph.

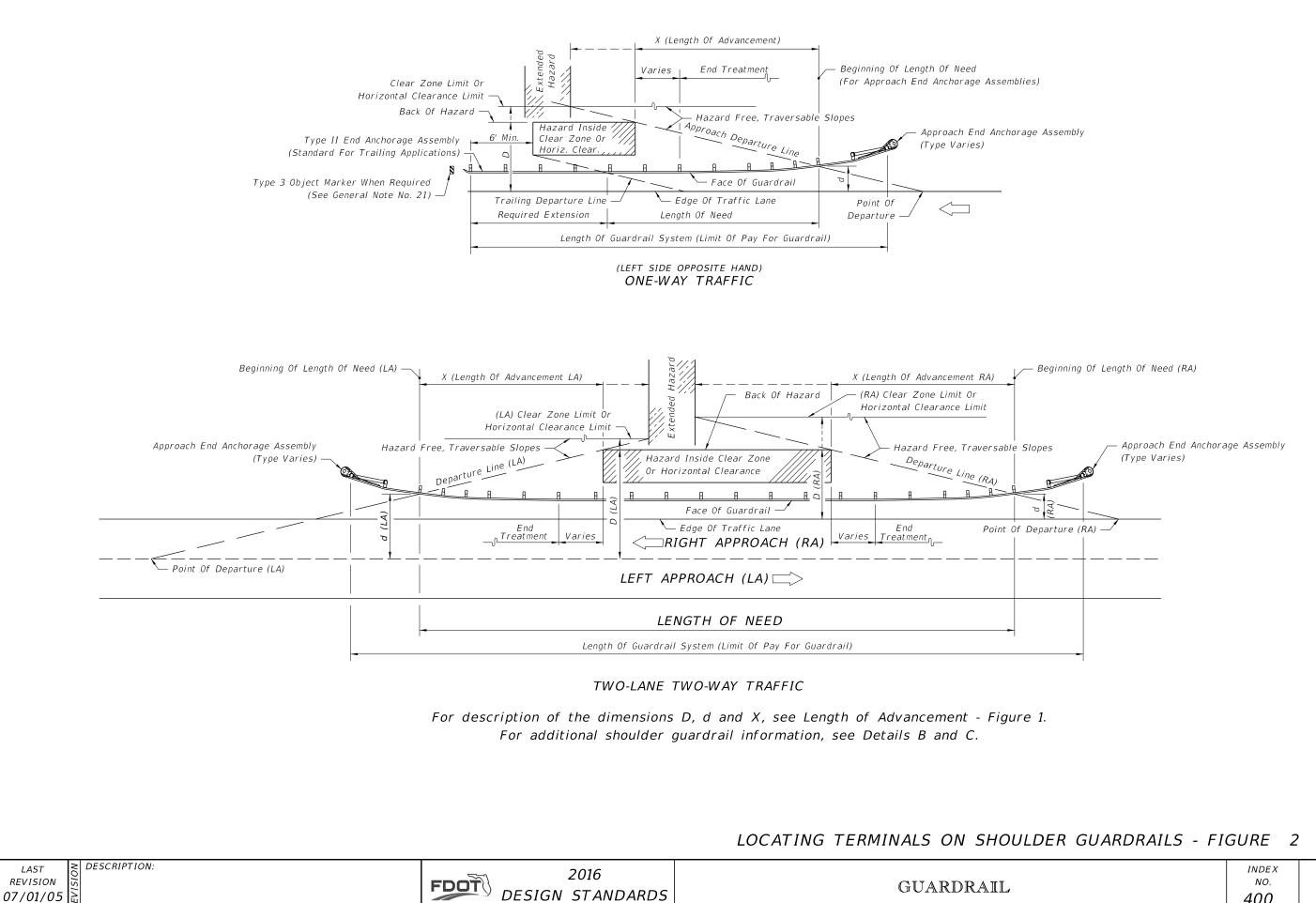
LAST REVISION 07/01/09



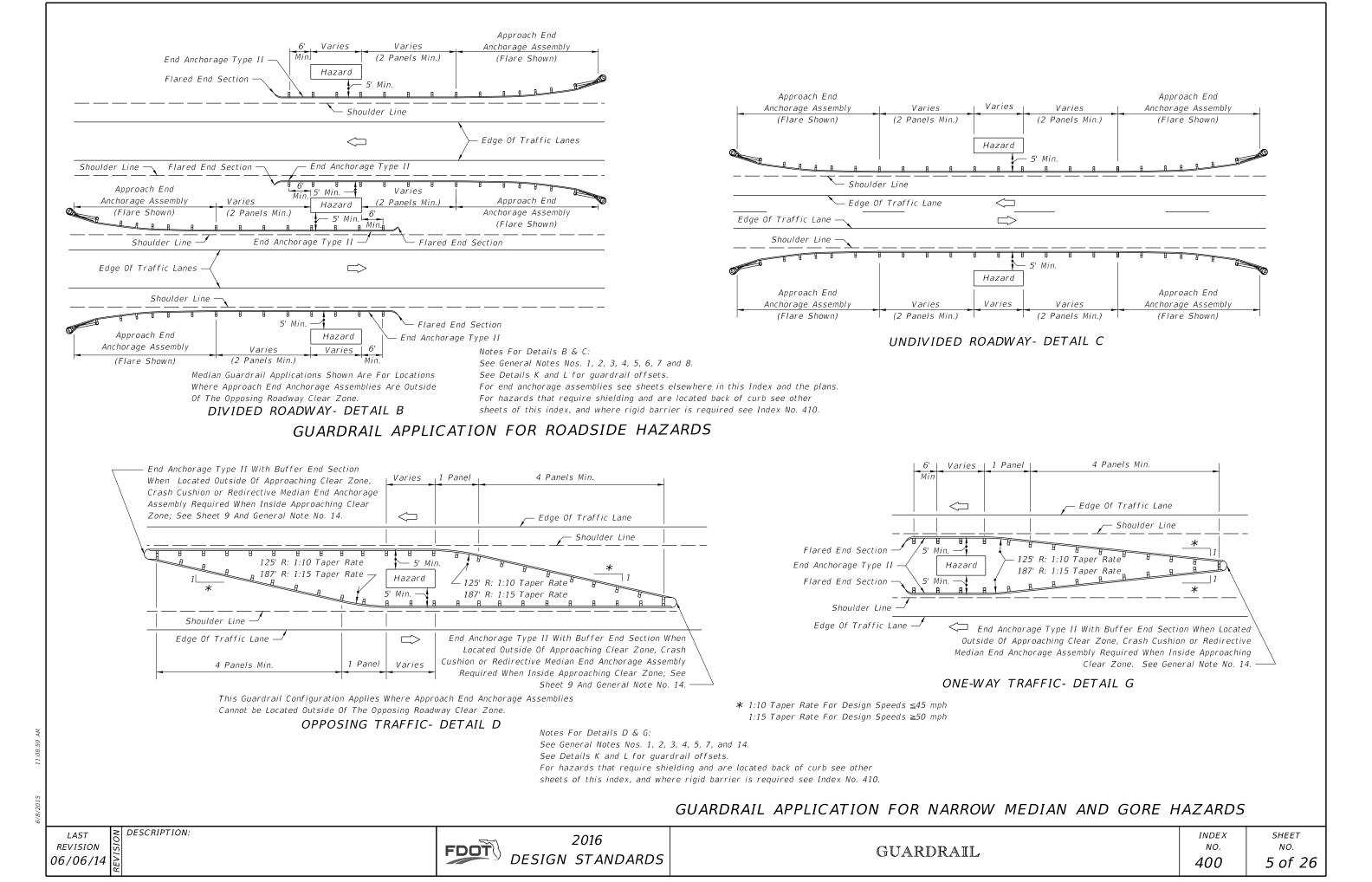
GUARDRAIL

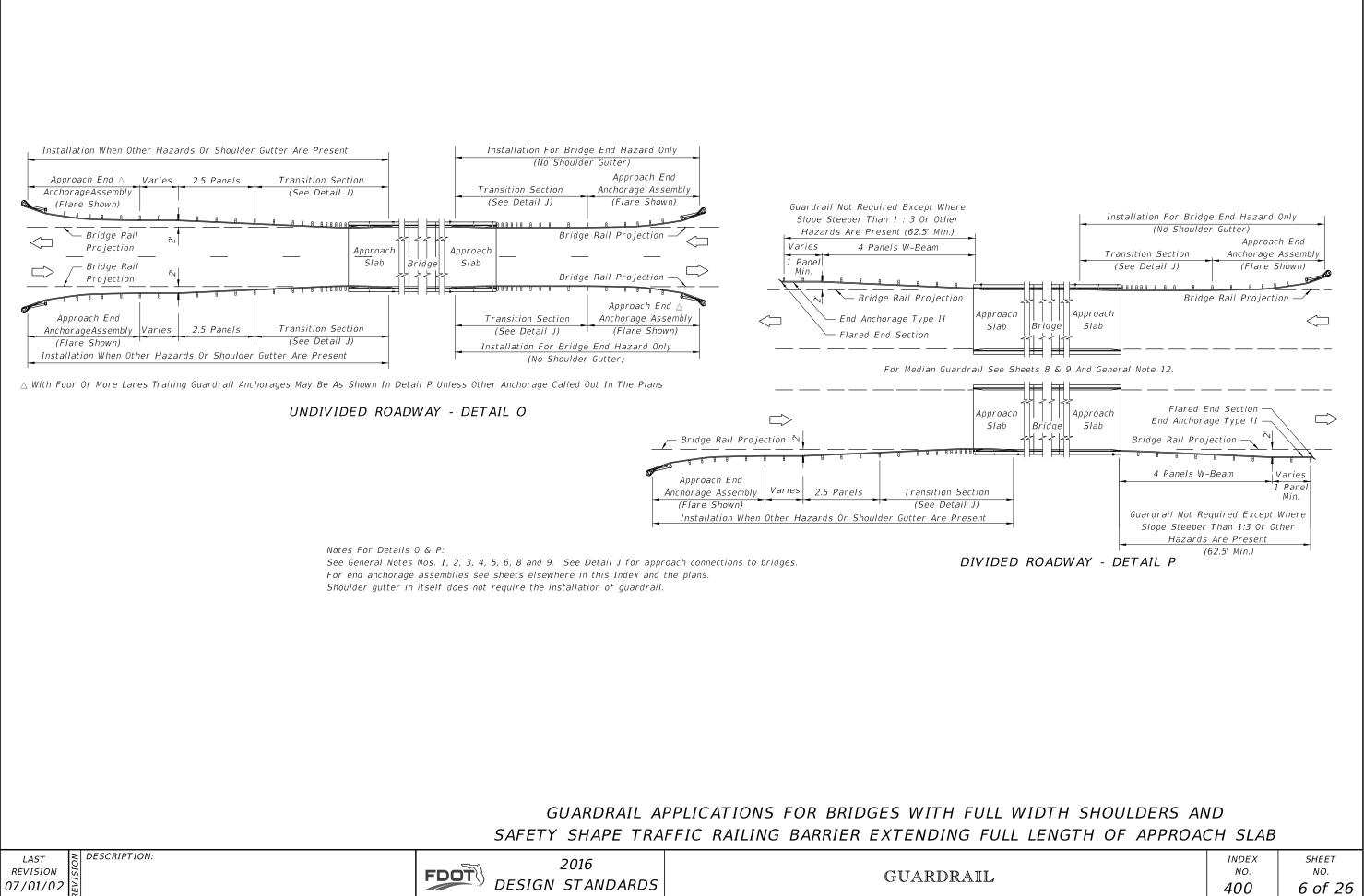
# LENGTH OF ADVANCEMENT - FIGURE 1

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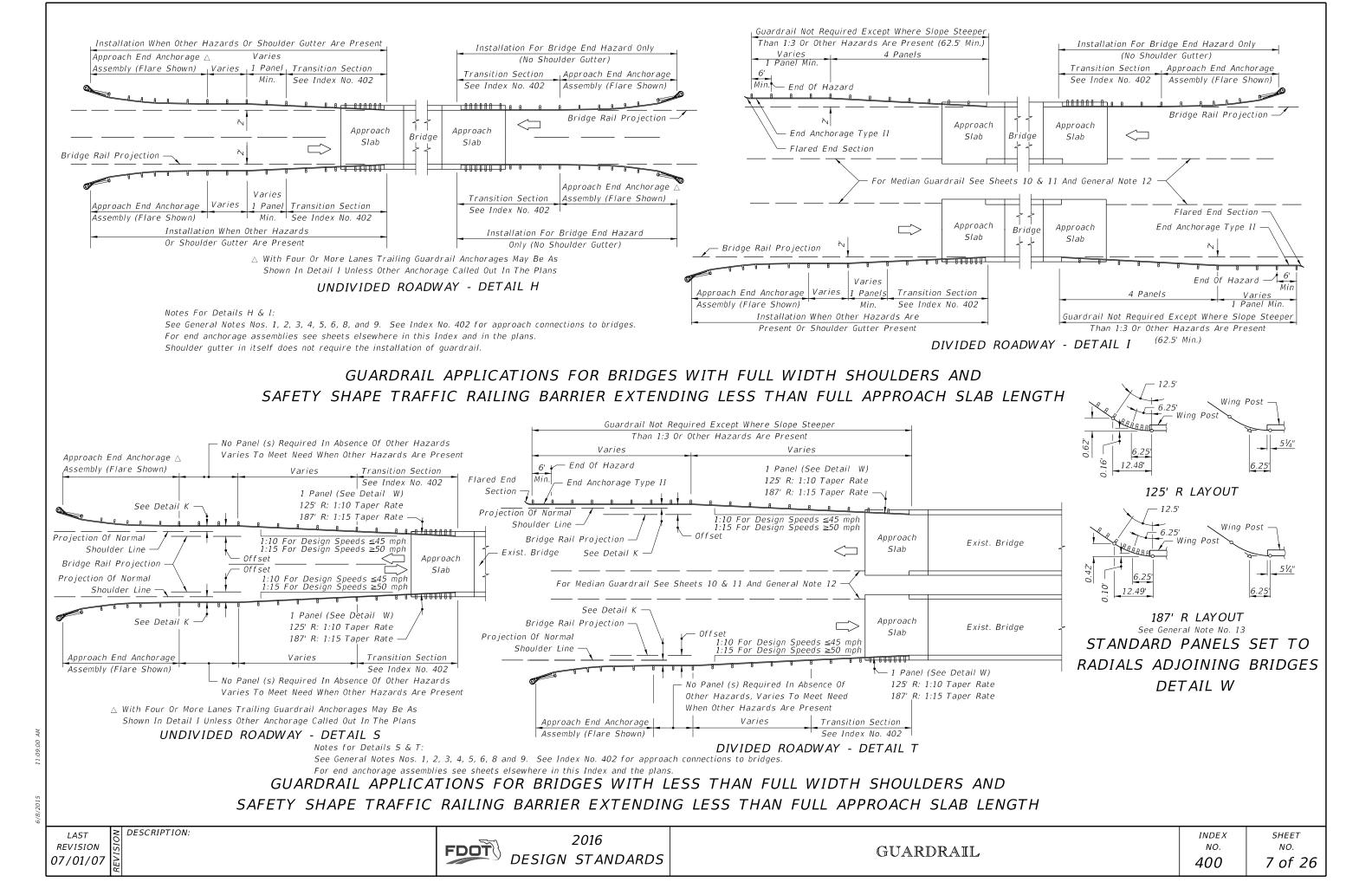


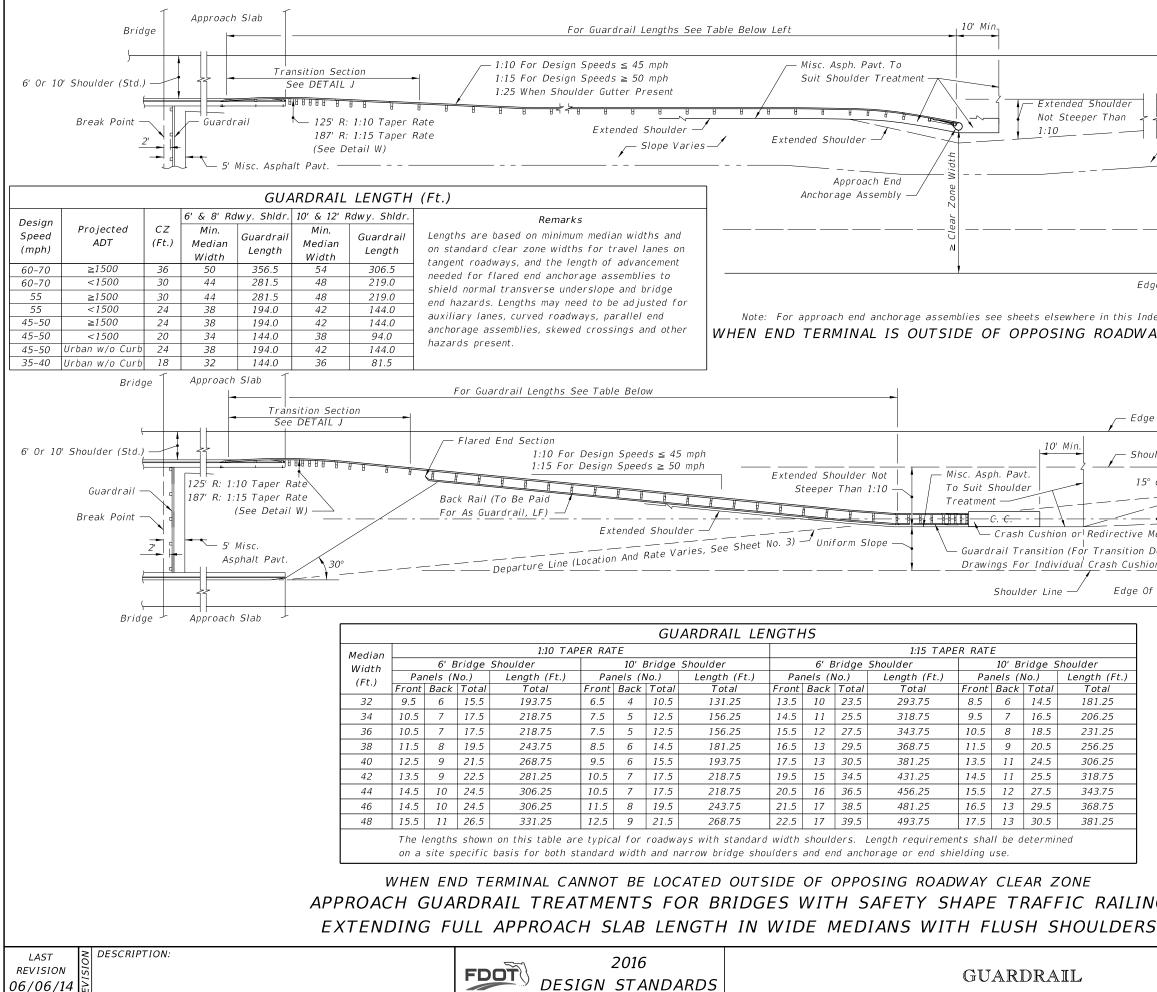
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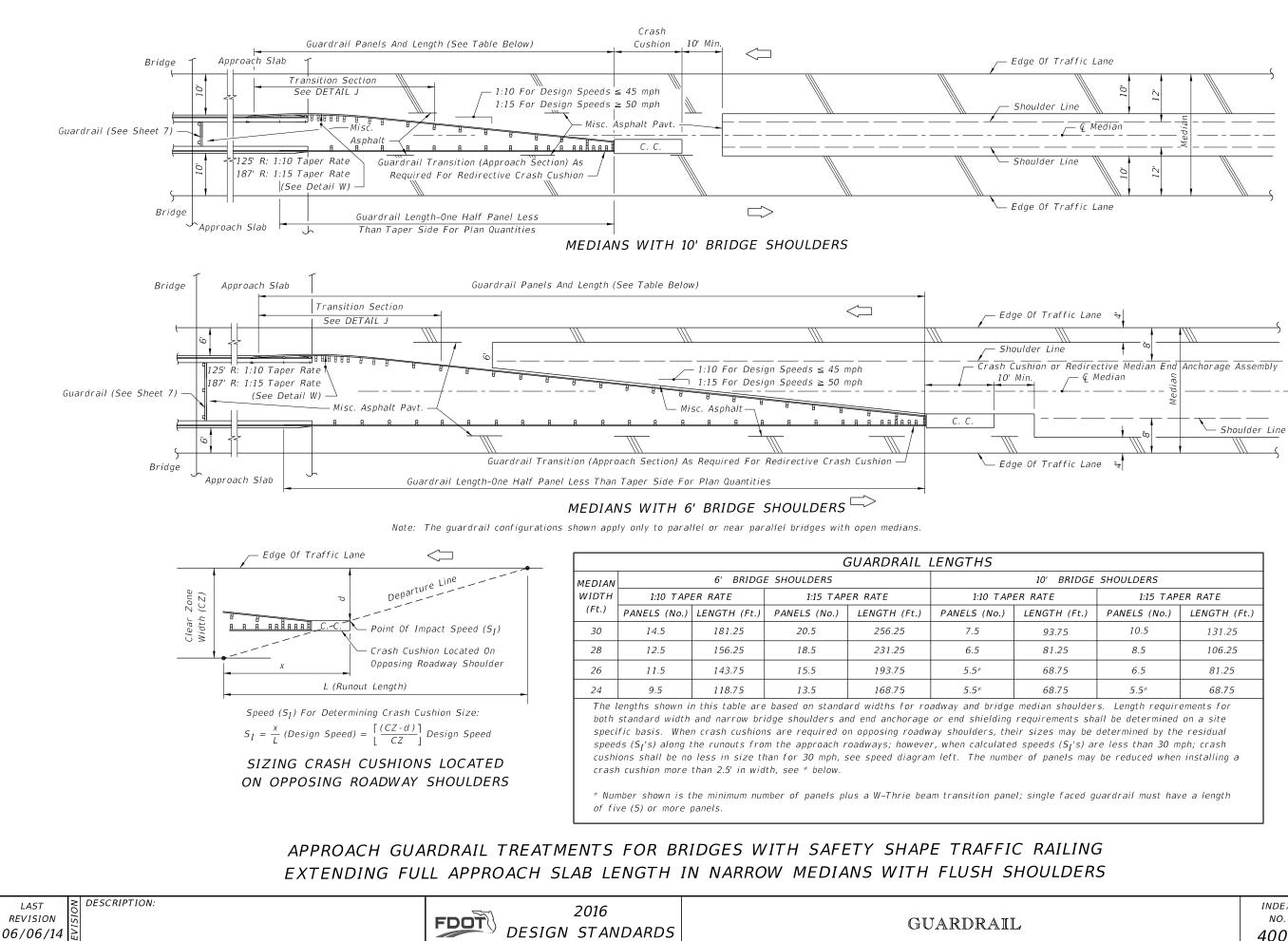


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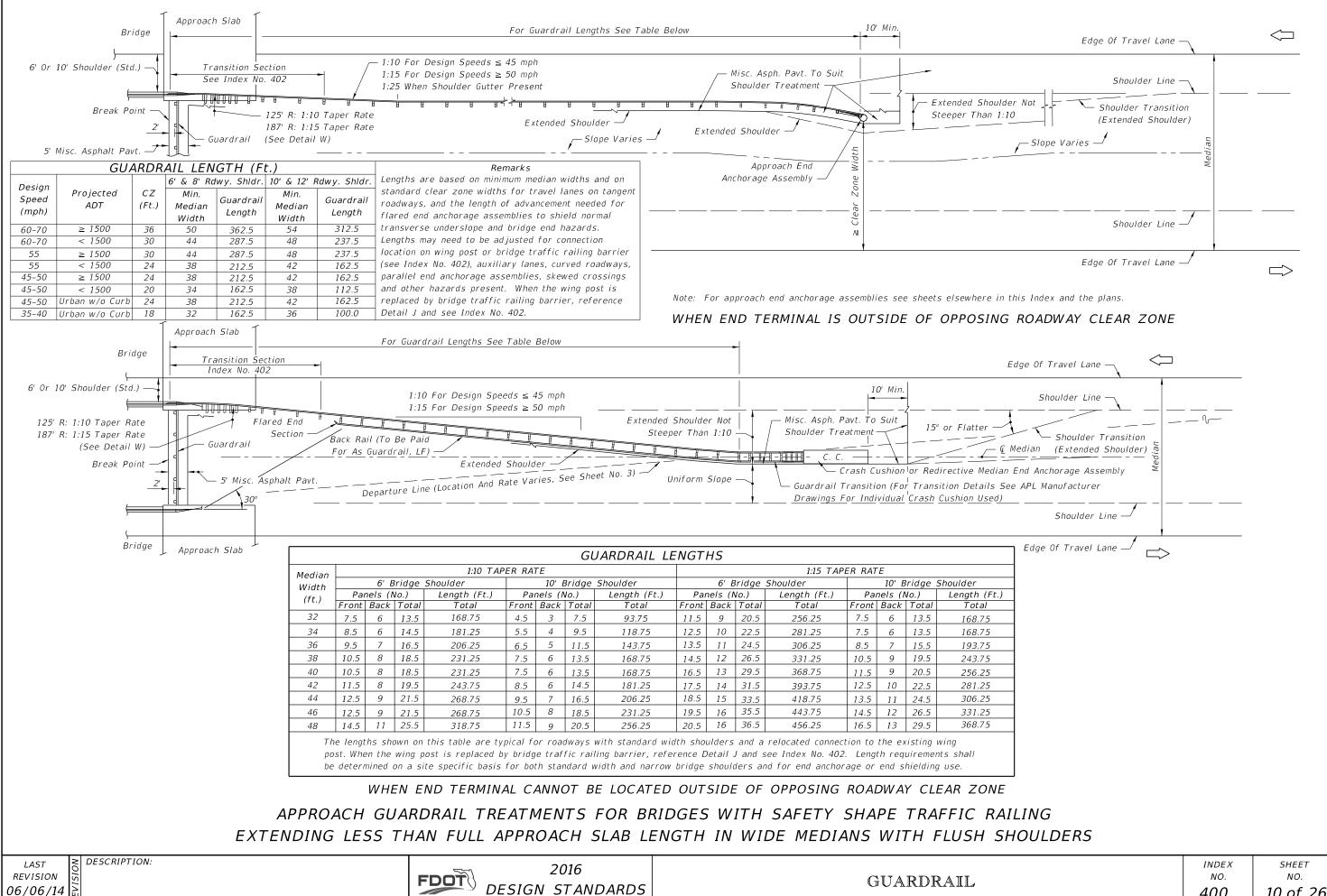
Edge Of Trave	l Lane	$\bigcirc$
Shoulder Line — Shoulder Transitio (Extended Shoulder Slope Varies —		
Shoulder Line		
lge Of Travel Lane —/ dex and the plans. AY CLEAR ZONE		
e Of Traffic Lane		Ŧ
	Transition Shoulder)	11-CO1011
f Traffic Lane —		<u> </u>
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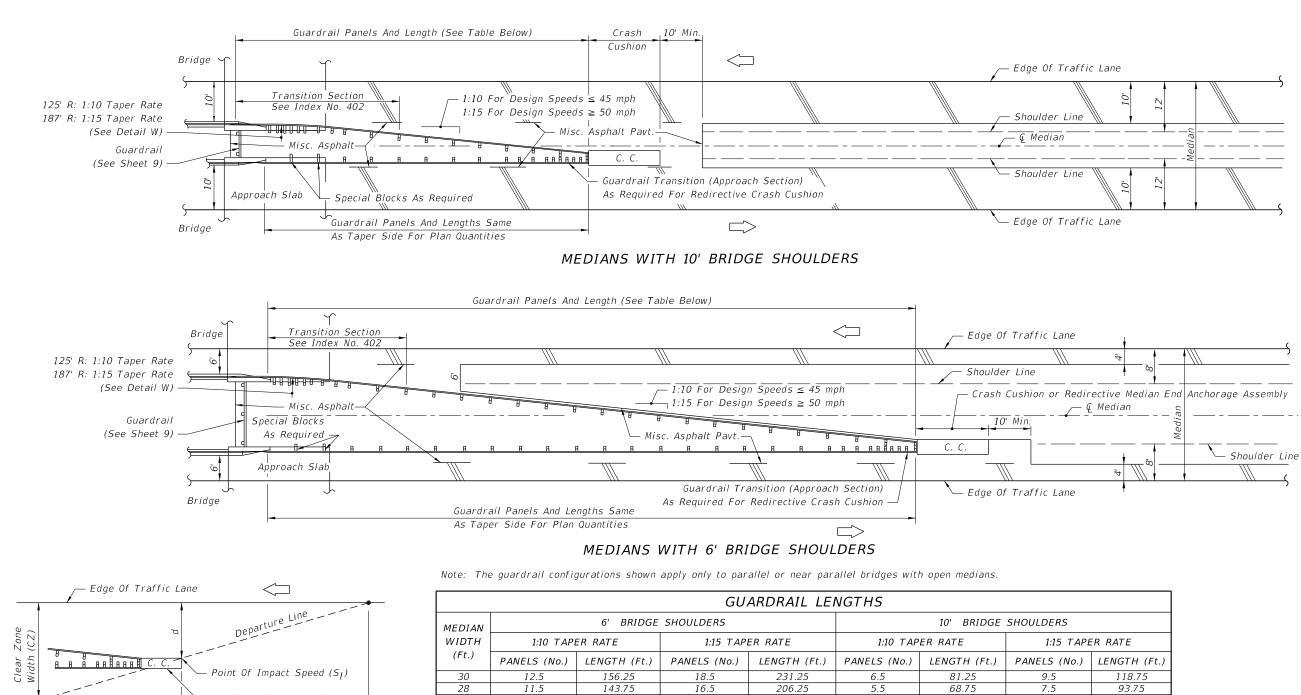
ΞE	SHOULDERS			
1:15 TAPER RATE				
)	PANELS (No.)	LENGTH (Ft.)		
	10.5	131.25		
	8.5	106.25		
	6.5	81.25		
	5.5*	68.75		

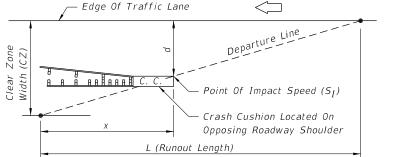
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Speed (S<sub>1</sub>) For Determining Crash Cushion Size:  $S_I = \frac{x}{L} (Design Speed) = \left[\frac{(CZ-d)}{CZ}\right] Design Speed$ SIZING CRASH CUSHIONS LOCATED

ON OPPOSING ROADWAY SHOULDERS

	GUARDRAIL LENGTHS						
MEDIAN WIDTH (Ft.)	6' BRIDGE SHOULDERS				10' BRIDGE	SHOULDERS	
	1:10 TAP	ER RATE	1:15 TAPER RATE 1:10 TAPER		ER RATE	1:15 TAP	
	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)
30	12.5	156.25	18.5	231.25	6.5	81.25	9.5
28	11.5	143.75	16.5	206.25	5.5	68.75	7.5
26	9.5	118.75	14.5	181.25	5.5*	68.75	5.5*
24	8.5	106.25	11.5	143.75	5.5*	68.75	5.5*

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirement standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds ( $S_{I}$ 's) along the runouts from the approach roadways; however, when calculated speeds ( $S_I$ 's) are less than 30 mph crash cushions shall be no less in size than for 30 mph; see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see \* below.

\*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHO

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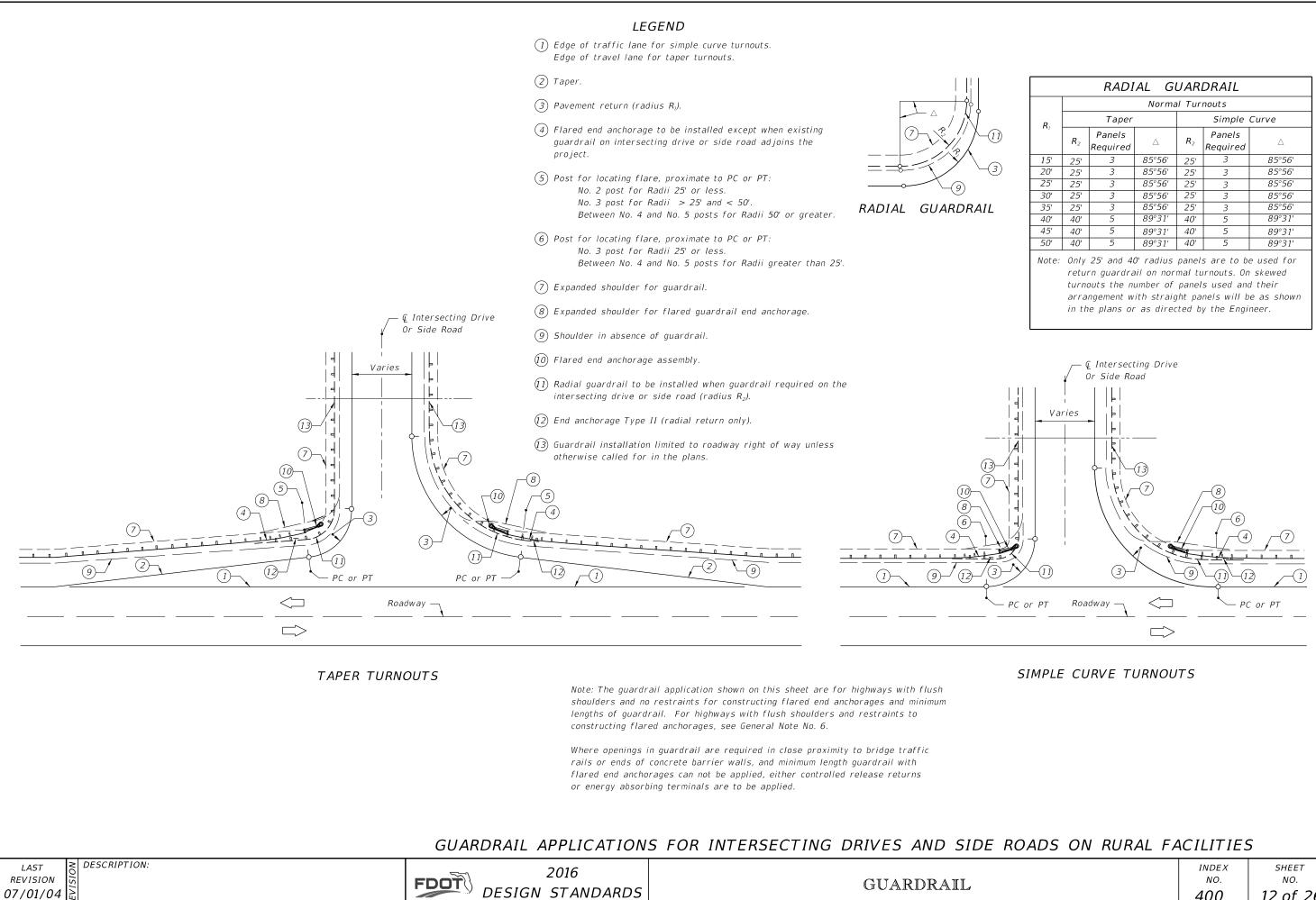


2016 DESIGN STANDARDS

GUARDRAIL

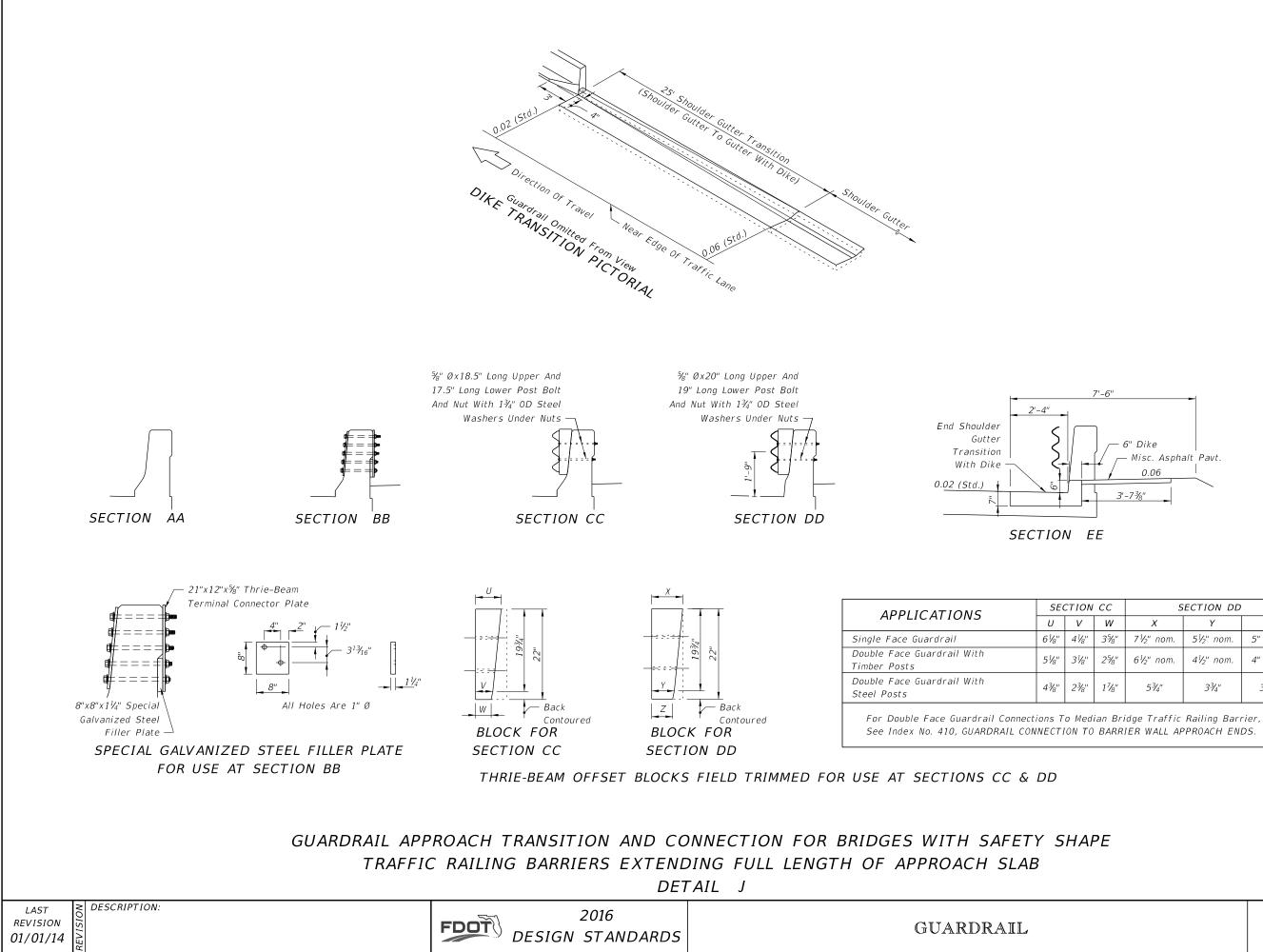
PE	R RATE
	LENGTH (Ft.)
	118.75
	93.75
	68.75
	68.75
5	for both

OULDERS		
	index no. <b>400</b>	<sup>SHEET</sup> NO. <b>11 of 26</b>



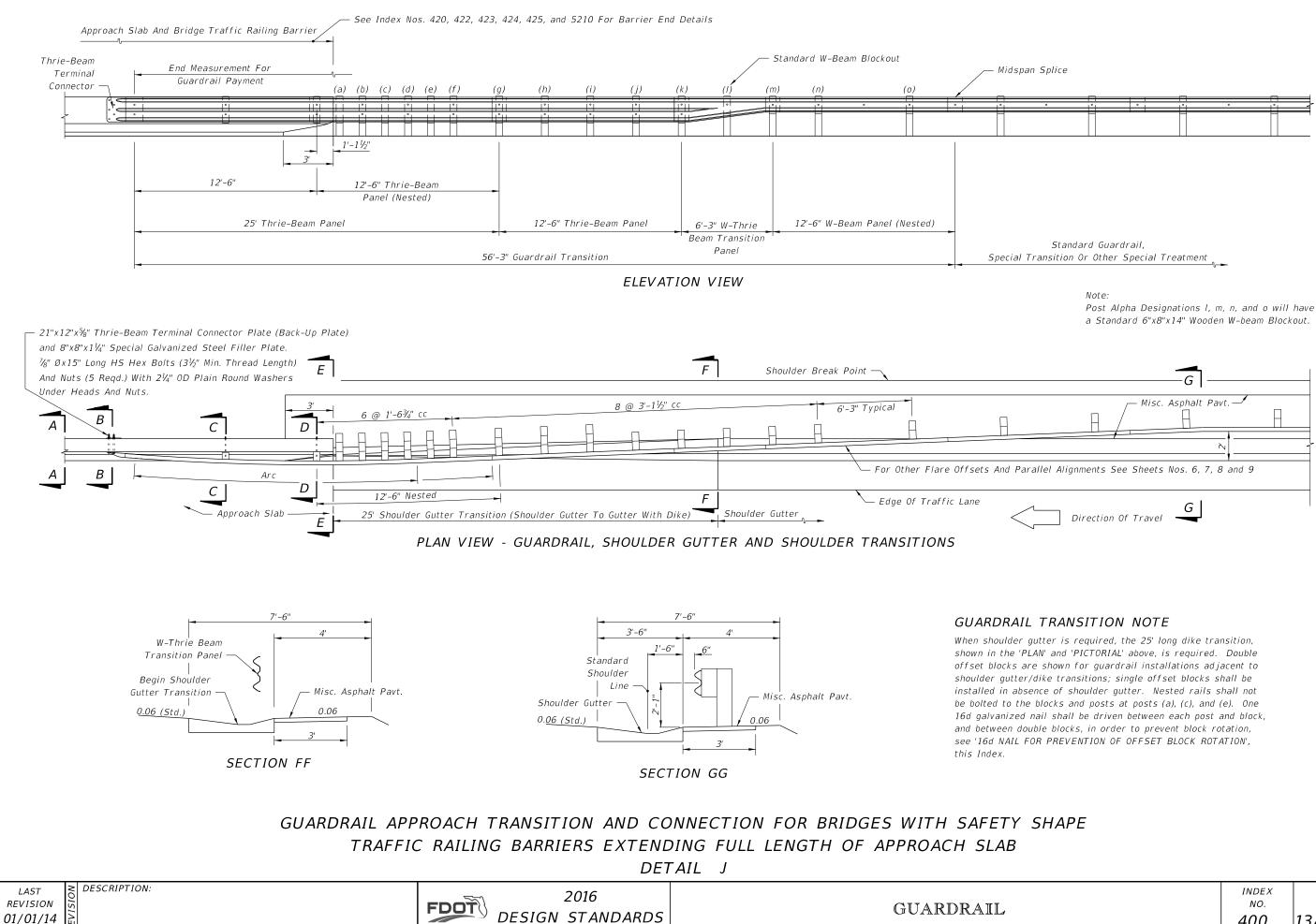
RADIAL GUARDRAIL								
			Norma	al Tur	nouts			
$R_{i}$	Taper Simple Curve							
$\mathbf{n}_{1}$	<i>R</i> <sub>2</sub>	Panels Required	Δ	<i>R</i> <sub>2</sub>	Panels Required	Δ		
15'	25'	3	85°56'	25'	3	85°56'		
20'	25'	3	85°56'	25'	3	85°56'		
25'	25'	3	85°56′	25'	3	85°56'		
30'	25'	3	85°56'	25'	3	85°56'		
35'	25'	3	85°56'	25'	3	85°56'		
40'	40'	5	89°31'	40'	5	89°31'		
45'	40'	5	89°31'	40'	5	89°31'		
50'	40'	5	89°31'	40'	5	89°31'		
Note: Only 25' and 40' radius panels are to be used for return guardrail on normal turnouts. On skewed turnouts the number of panels used and their								
	arra	ngement w	ith straig	ght pa	nels will b	e as shown		
	in the plans or as directed by the Engineer.							

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I	СС	S	ECTION DD	
	W	X	Y	Ζ
	35⁄8″	7½" nom.	5½" nom.	5" nom.
	25/8"	6½" nom.	4½" nom.	4" nom.
	17/8"	5 <sup>3</sup> ⁄4″	3 <sup>3</sup> /4"	3¼"

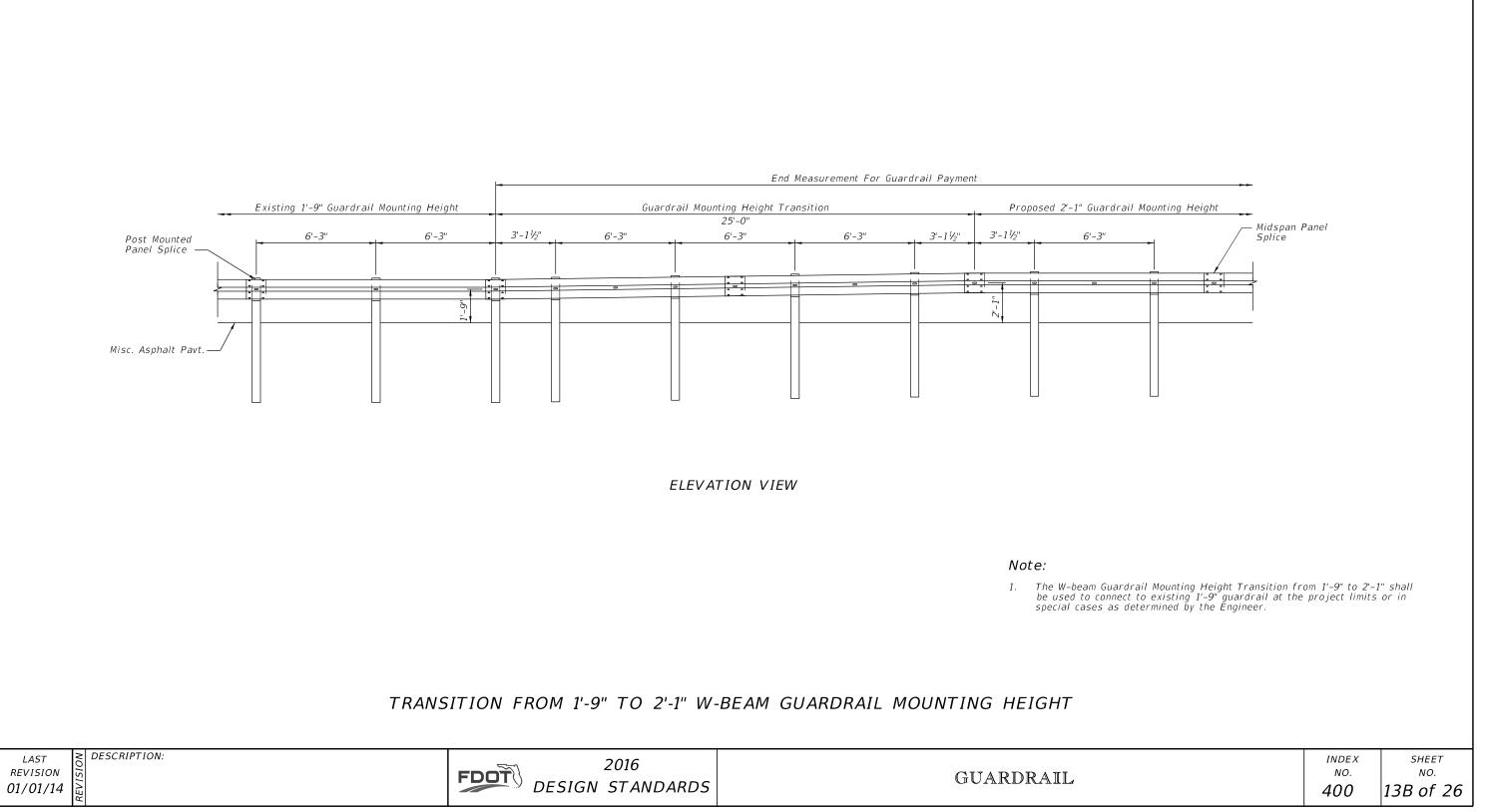
INDEX	SHEET
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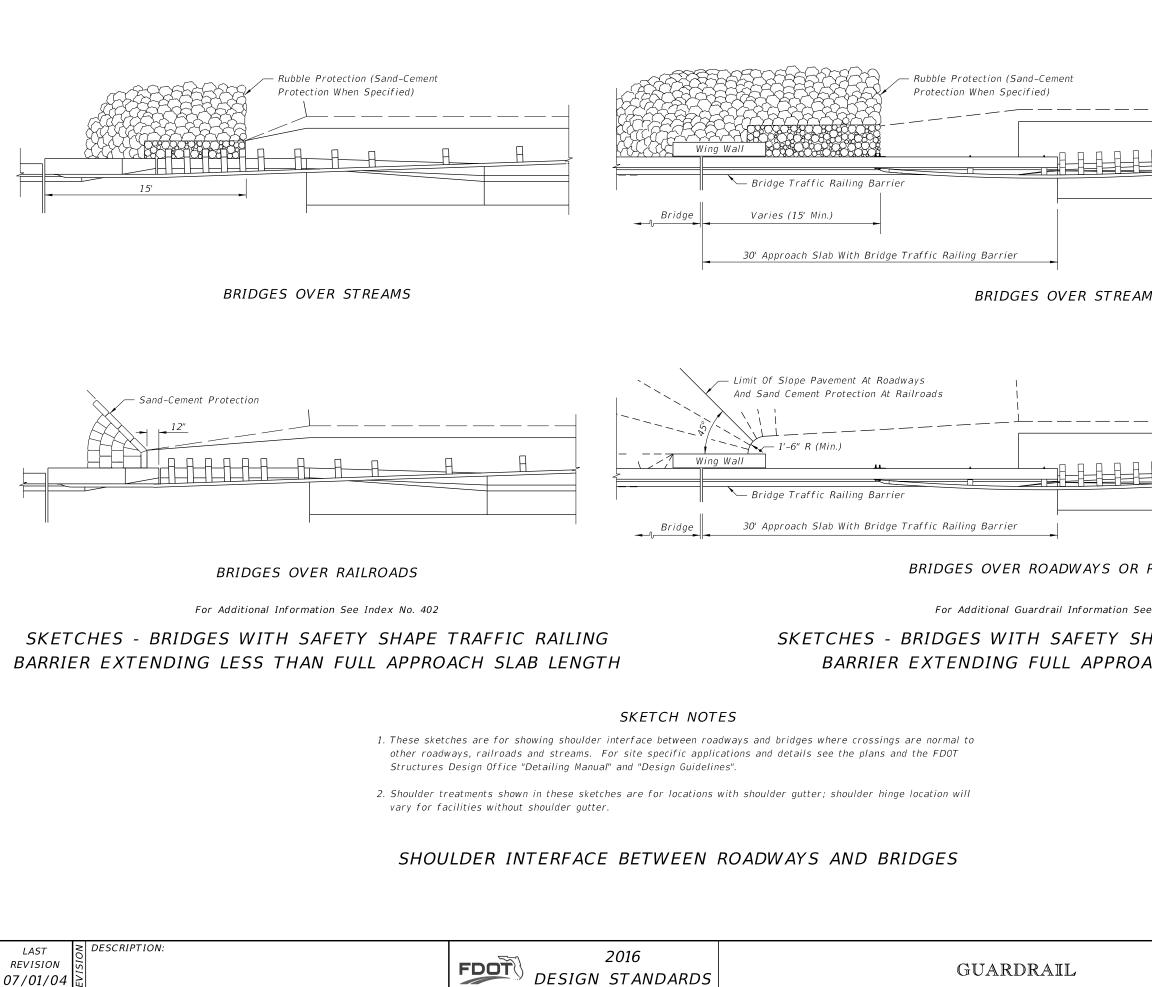


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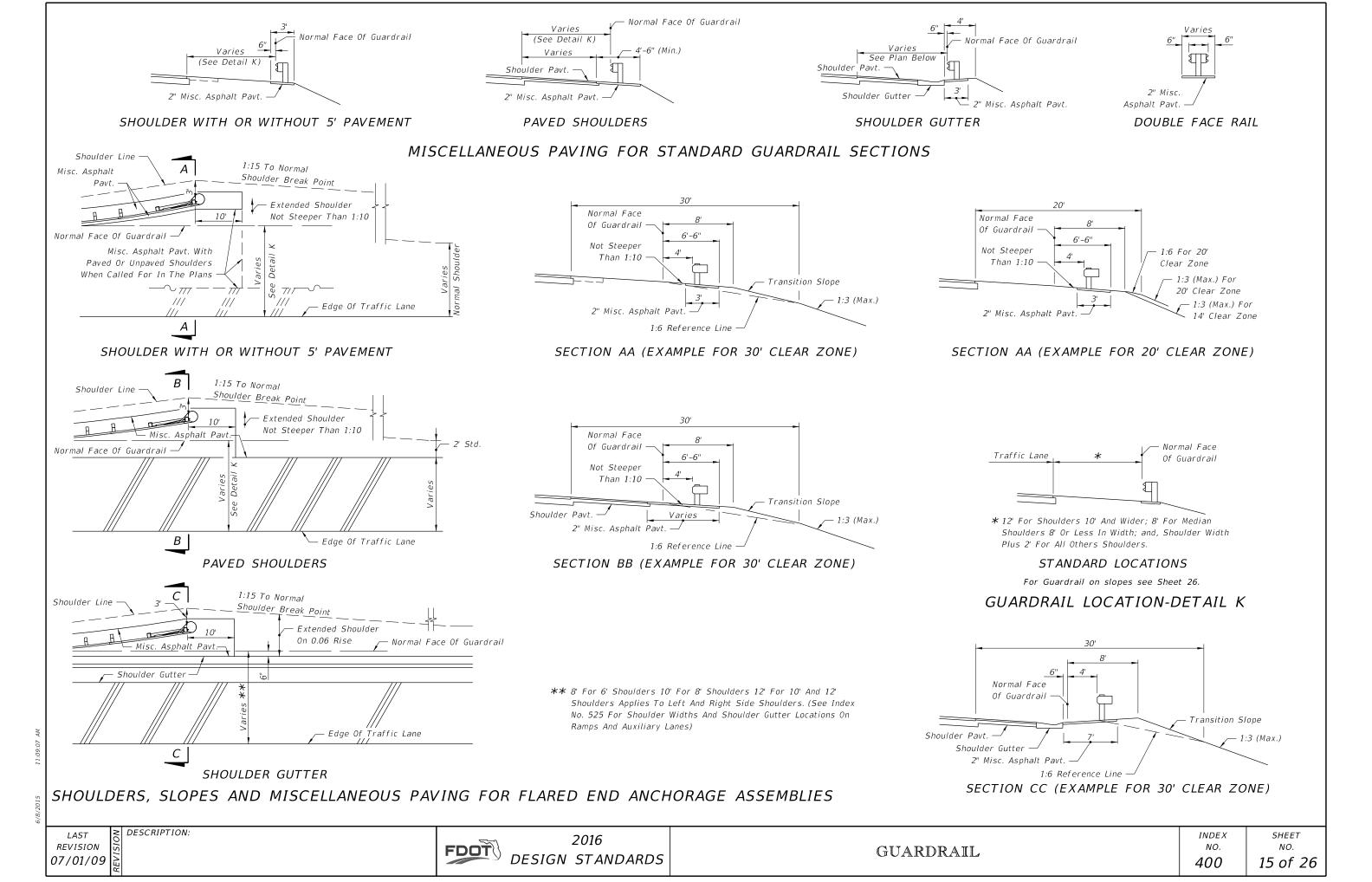
ndard	Guardrai	Ι,		
n Or O	ther Spe	cial Trea	atment	

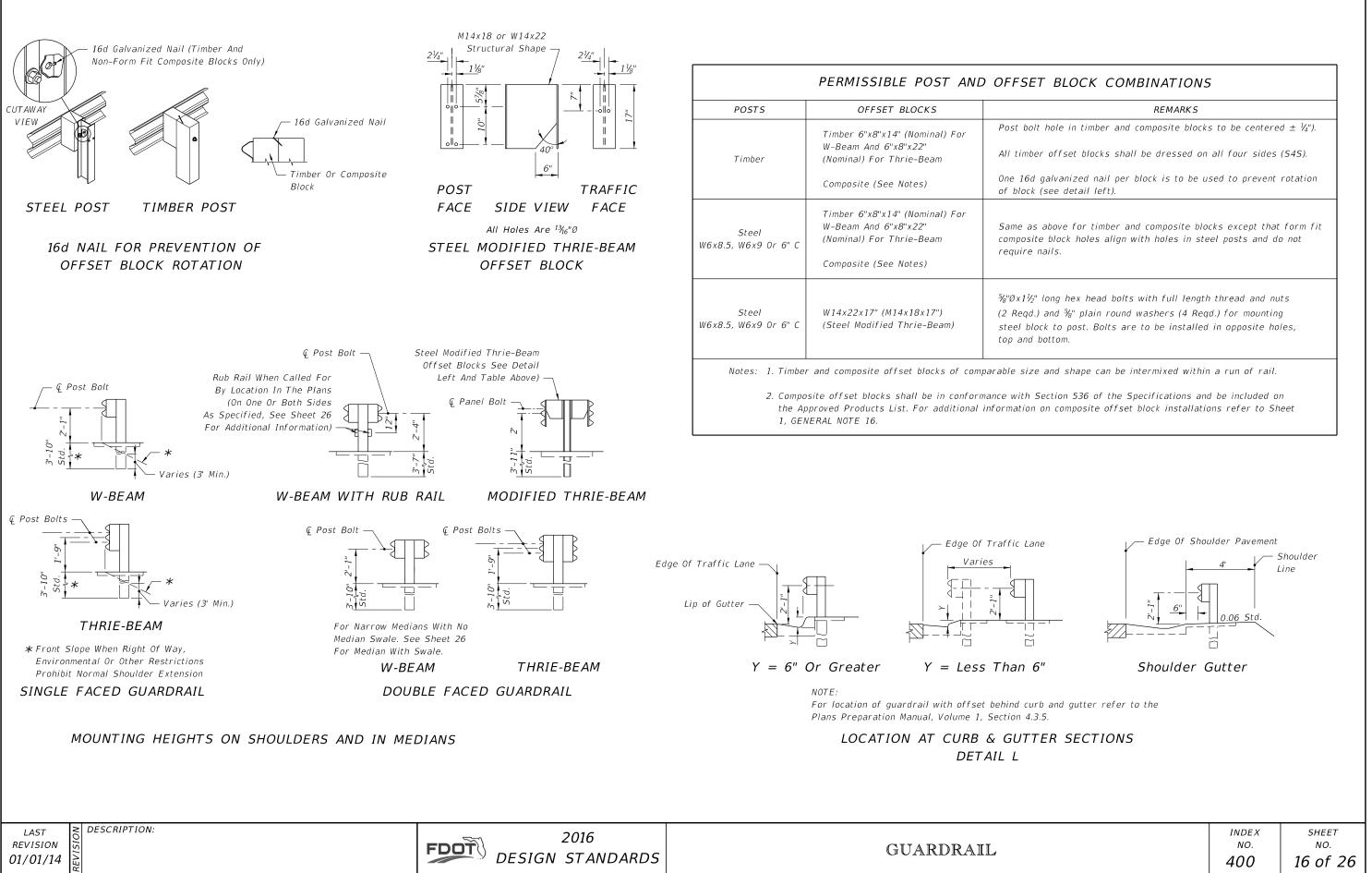
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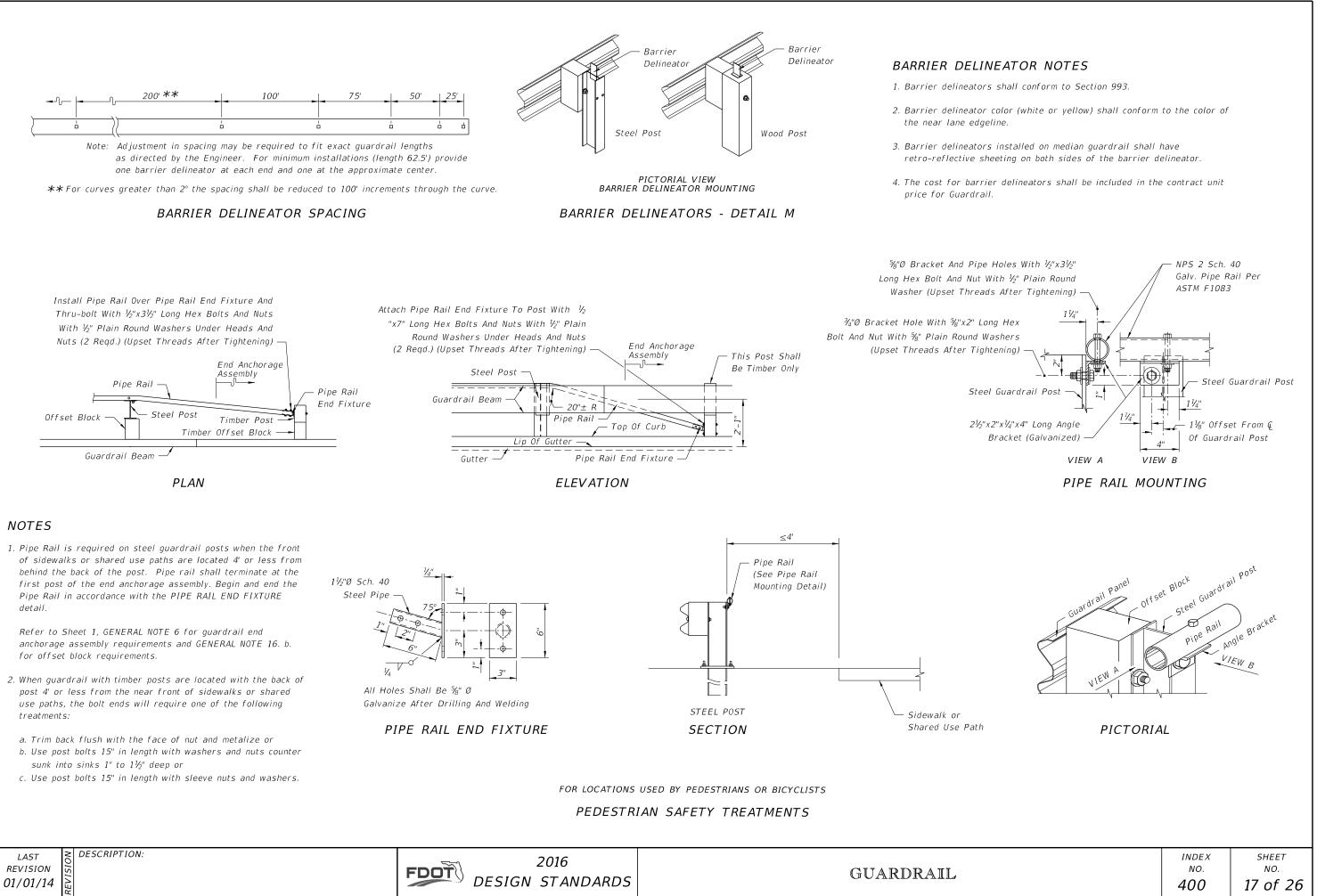




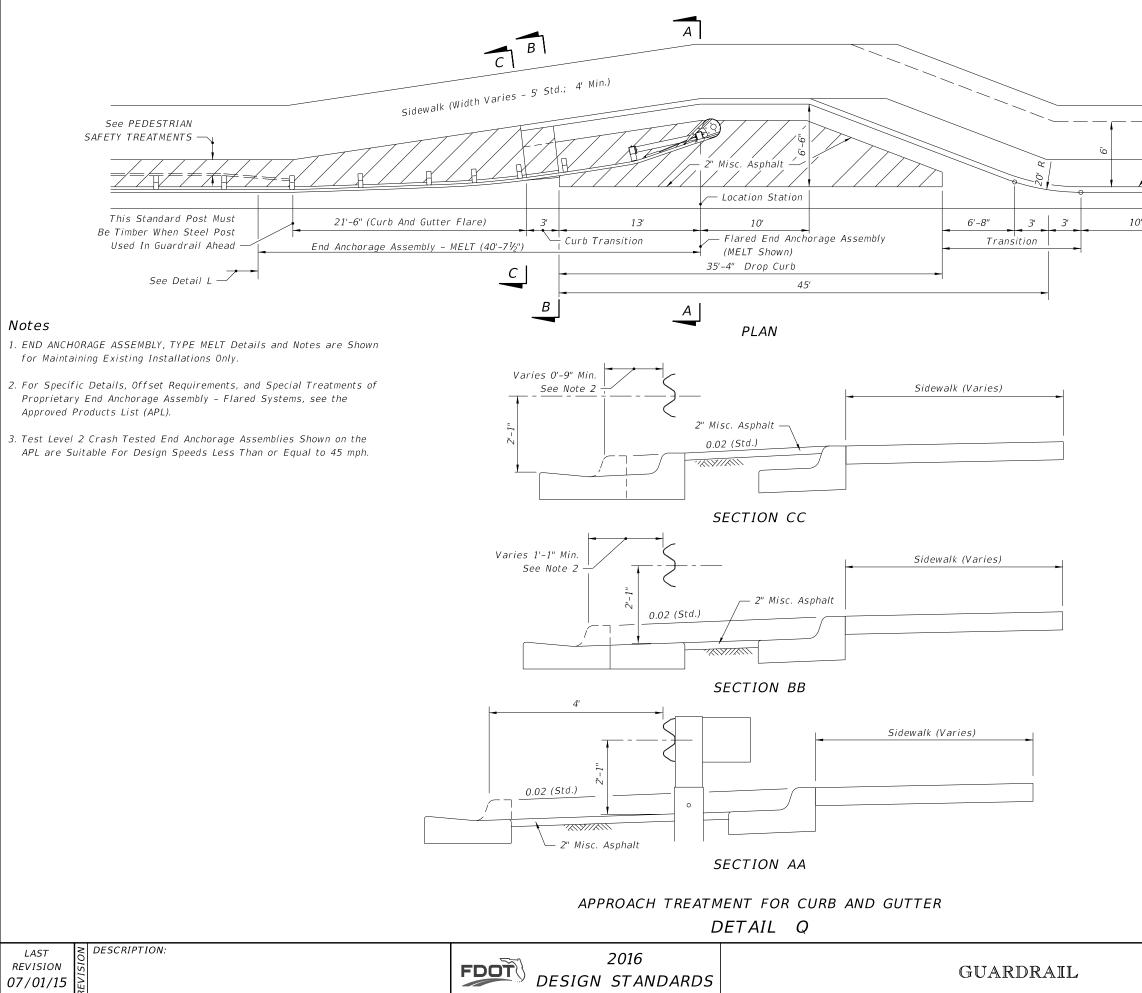
45							
				<u> </u>			
RAIL	ROAL	55					
ΗAP			FIC R LENGT	AILING ⁻H			
				INDE	<hr/>	SHEET	



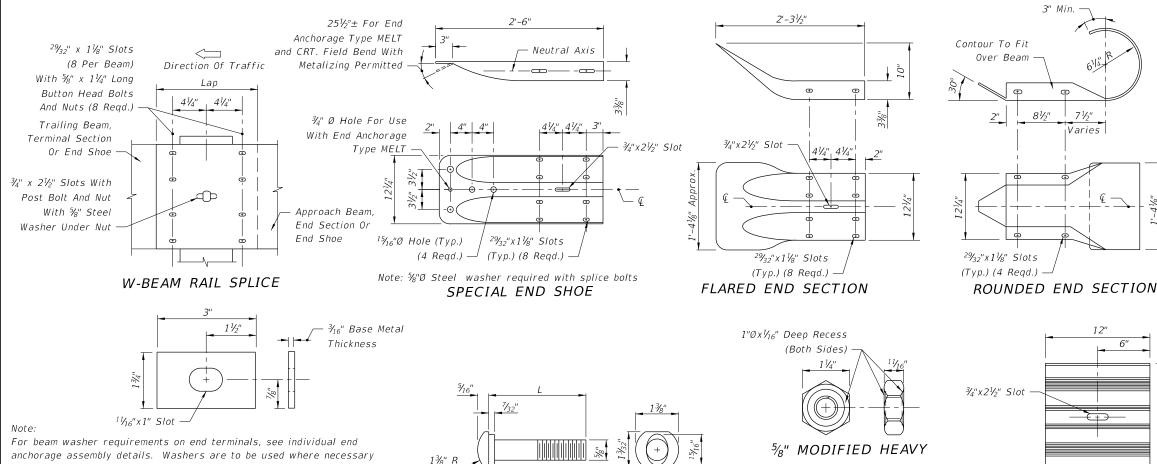




REVISION



— Sidewalk Without Utility S	strip _	
	,	
<i>i</i>		
, Curb And G	utter Type F	
4		
	<b>I</b>	
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Note: For application information see individual

end anchorage assembly details.

### W-BEAM BACK-UP PLATE

		OFFSETS (Ft.)			
Measured From Face Of Guardrail To Of Above Ground Rigid Hazard					nt
	POST	-	BEAMS	NESTED BEAM	
	SPACING (Ft.)	W-Beam	Thrie-Beam	W-Beam	Thrie-L
	6'-3"	5'-0"	3'-10"	N/A	N//
	3'-1½"	3'-10"	3'-2"	3'-0"	2'-1
	1'-6¾"	3'-2"	2'-10"	2'-8"	2'-6

Note:

HEX NUT (RECESSED NUT)

The values shown should be utilized unless changes are supported by empirical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5-6, "Summary of Maximum Delfections" of the AASHTO Roadside Design Guide are cautioned to proceed only if background in the table development is understood.

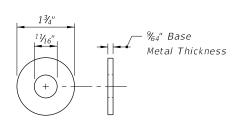
MINIMUM OFFSETS FOR SINGLE FACED GUARDRAIL (Ft.)

HS Hex bolts for THRIE-BEAM TERMINAL CONNECTORS shall conform to the requirements of ASTM A449 (Type 1) with heavy hex nuts and washers. All other hex bolts shall conform to the requirements of ASTM A563. Bolts, nuts and washers shall be hot dip galvanized. Heavy hex nut may be used in lieu of hex nuts and hex nuts used for jam nuts.

### HEX BOLTS AND NUTS

For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary to accomplish alignment or where the posts bolt head shows tendency to pull through the rail slot. Washers installed on guardrail, between end anchorages, prior to July 1, 1990 may remain in place until the guardrail is relocated or until repairs require removal and reinstallment of a post bolt.

## (RECTANGULAR PLATE WASHER) BEAM WASHER



### Note:

The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by %" Ø hex bolts and nuts and under hex nut for connecting rub rail to wood and steel posts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

# 5/8" STEEL WASHER

LAST	NC	DESCRIPTION
REVISION	SIC	
06/06/14	EVI	



organic zinc-rich coating.

THREAD LENGTH

(Min.) (In.)

⊿''

4"

⊿''

not less than 4".

Full Length Rail Splice Bolt

Post Bolt -

(In.)

1 1⁄4"

10"

18"

25"\*

APPLICATION

Single Or Double Faced Guardrail

Timber Or Composite Offset

As An Option, A Single 25"\* Long Post Bolt May Be Used

Double Faced Guardrail Steel Posts

Block(s) On Steel Post

Post Bolt - Single Faced Guardrail Timber Posts

Post Bolt - Double Faced Guardrail Timber Posts

Special bolts having lengths of 10" or greater shall have a thread length of

For applications where special bolts having lengths greater than 25" are

required, the Contractor may use a 5/8"Ø threaded rod (field cut to

no more than  $\frac{3}{4}$ " of the threaded rod projecting beyond the top of the nut.

both ends of the threaded rod metalized with organic zinc-rich coating.

the nut after pull-up shall be trimmed to  $\frac{3}{4}$ " reveal and metalized with

length). A hex nut and beam washer shall be used at the guardrail face with

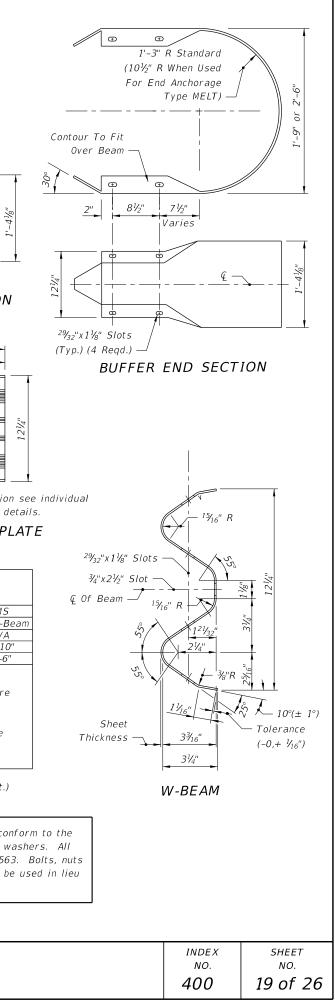
The projecting thread on both ends shall be distorted to secure the nuts, and

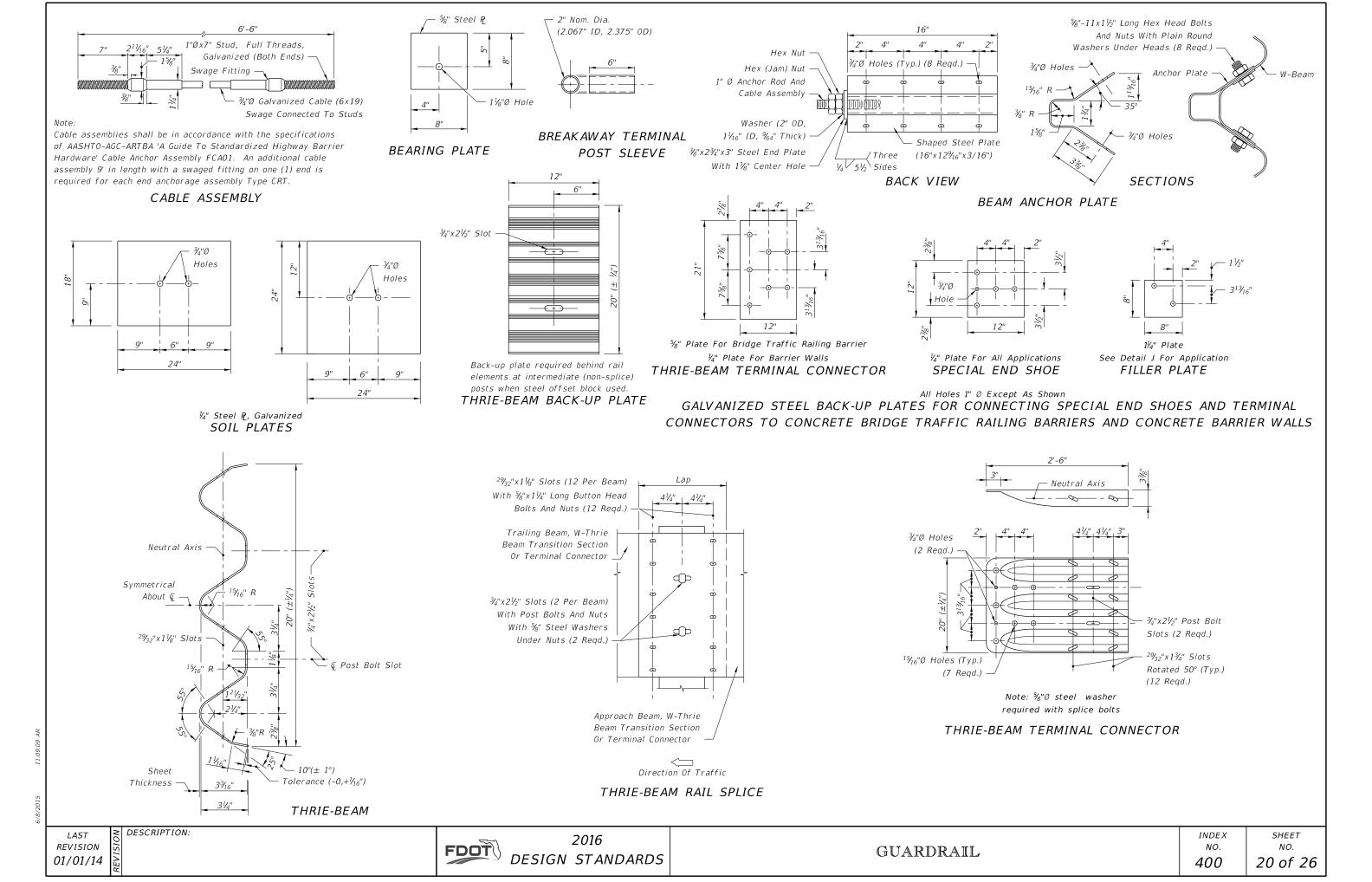
\*Use of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced

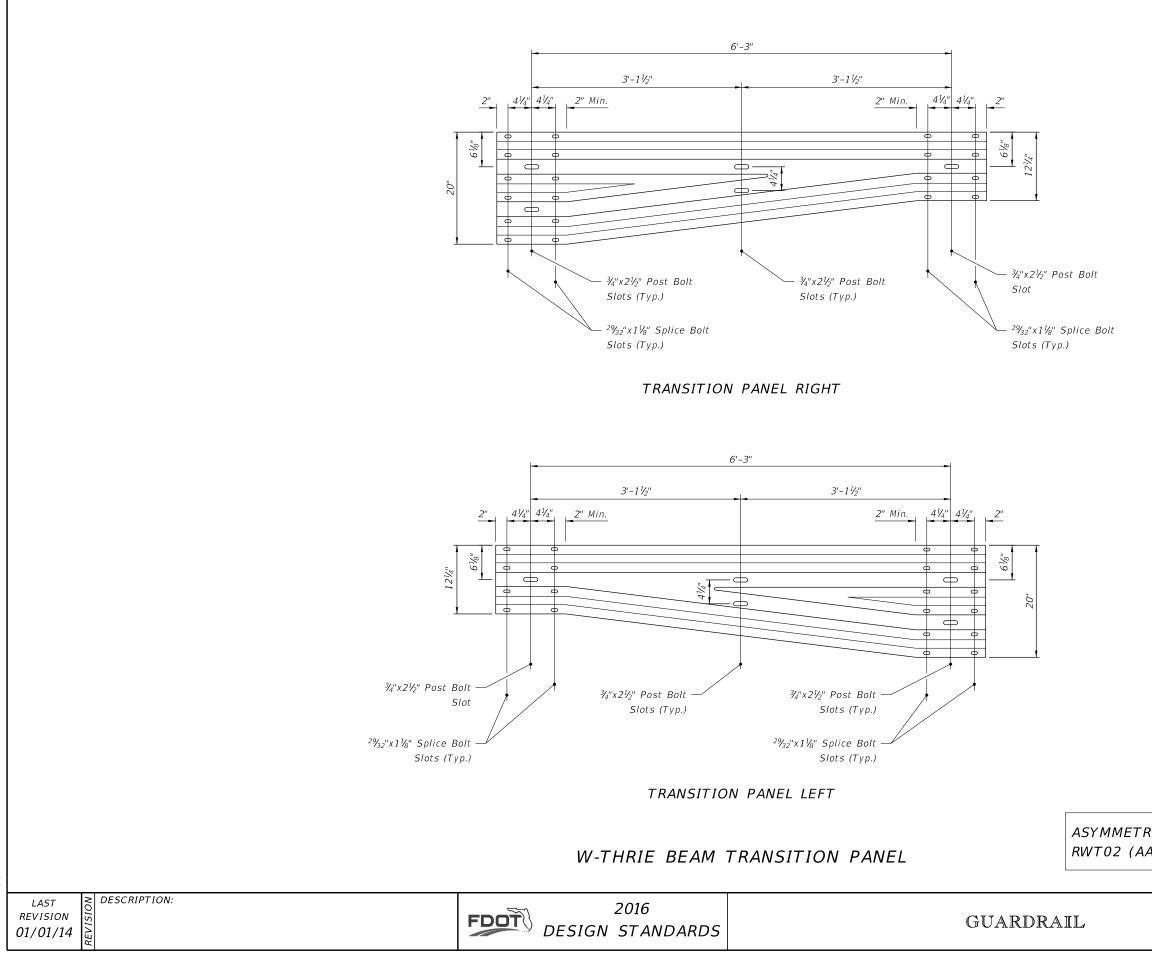
guardrail that results in the bolt projecting more than  $\frac{3}{4}$ " beyond the face of

 $\frac{5}{8}$ " OVAL SHOULDER BUTTON HEAD BOLT

GUARDRAIL

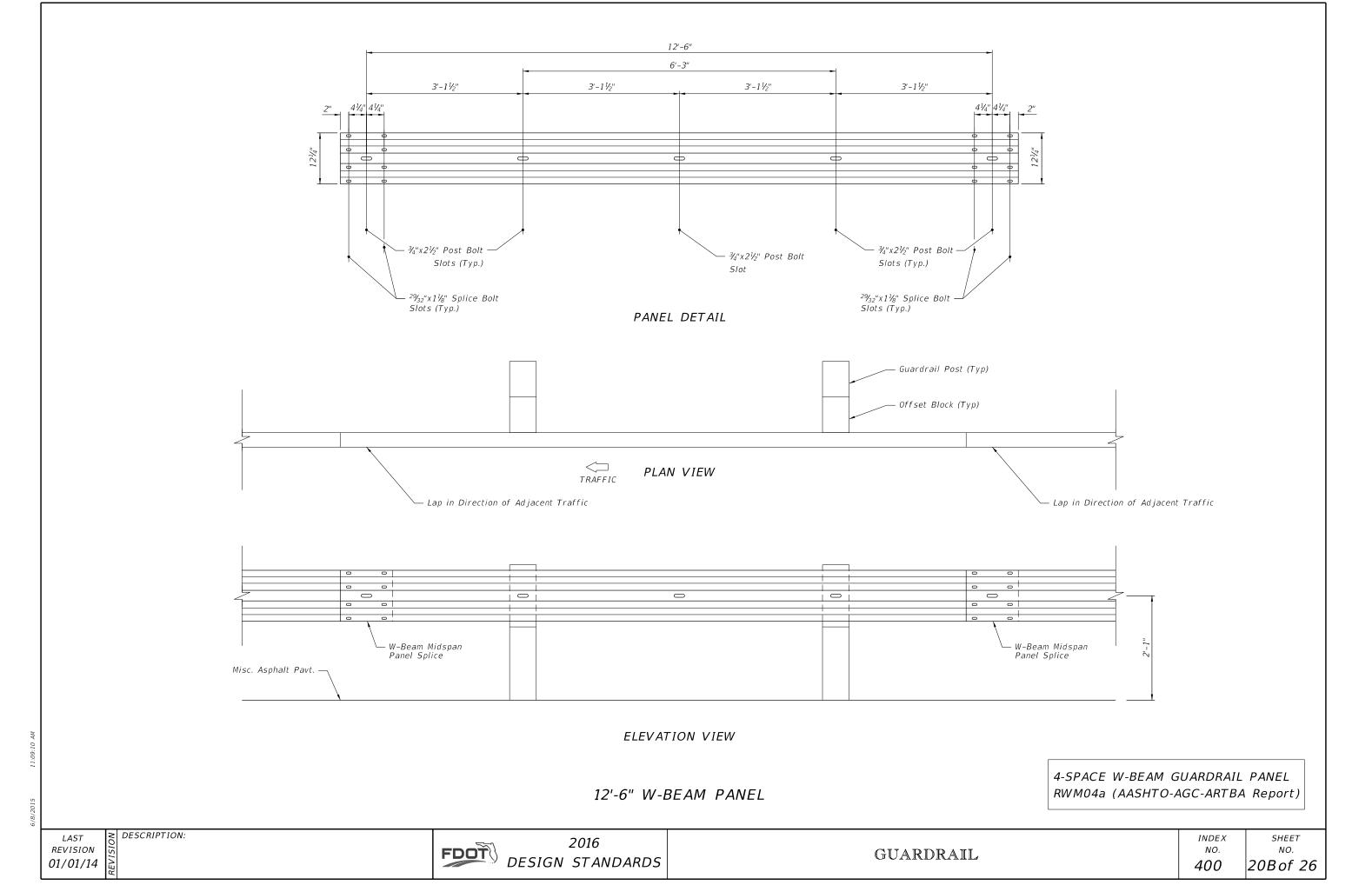


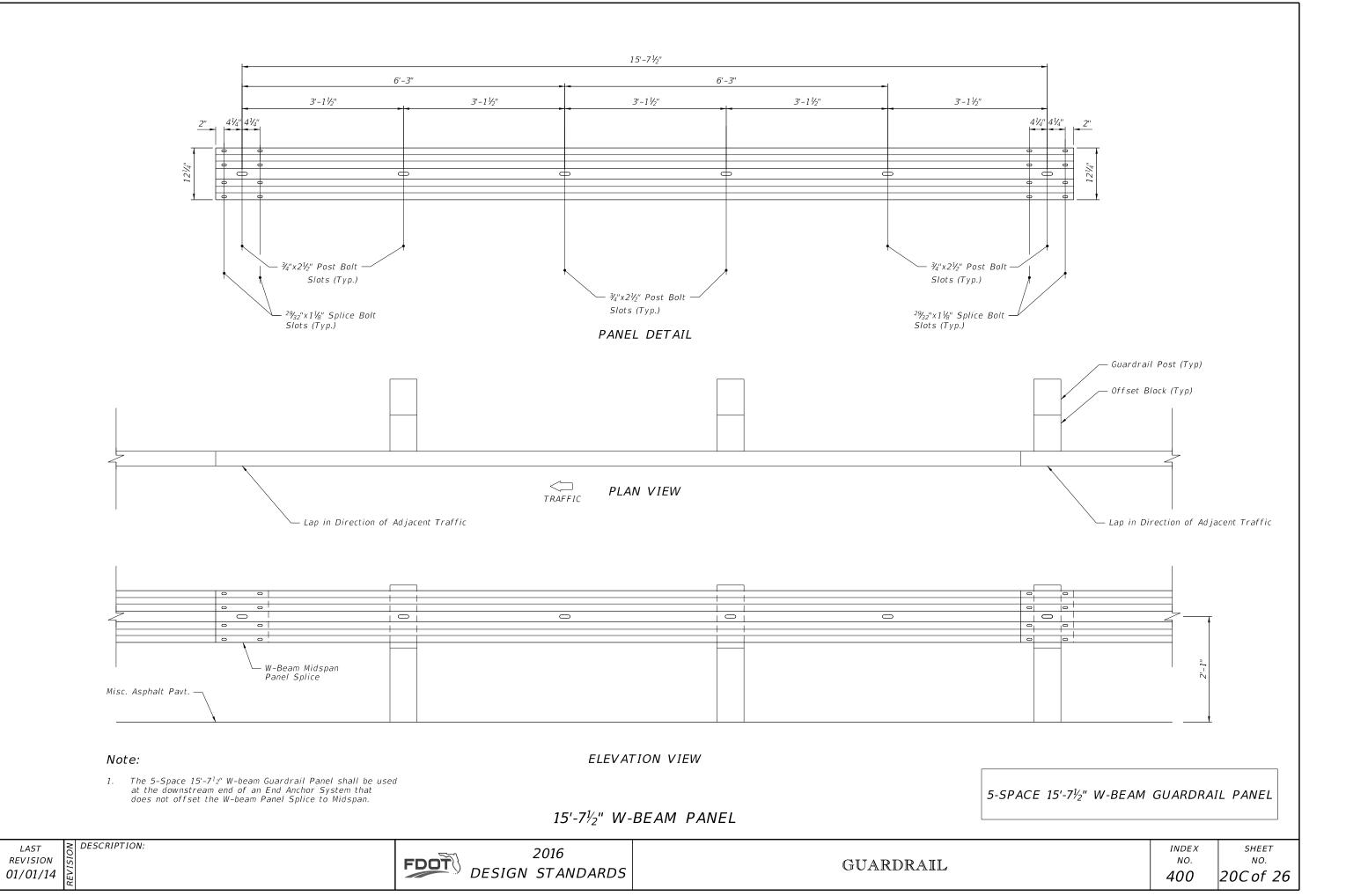


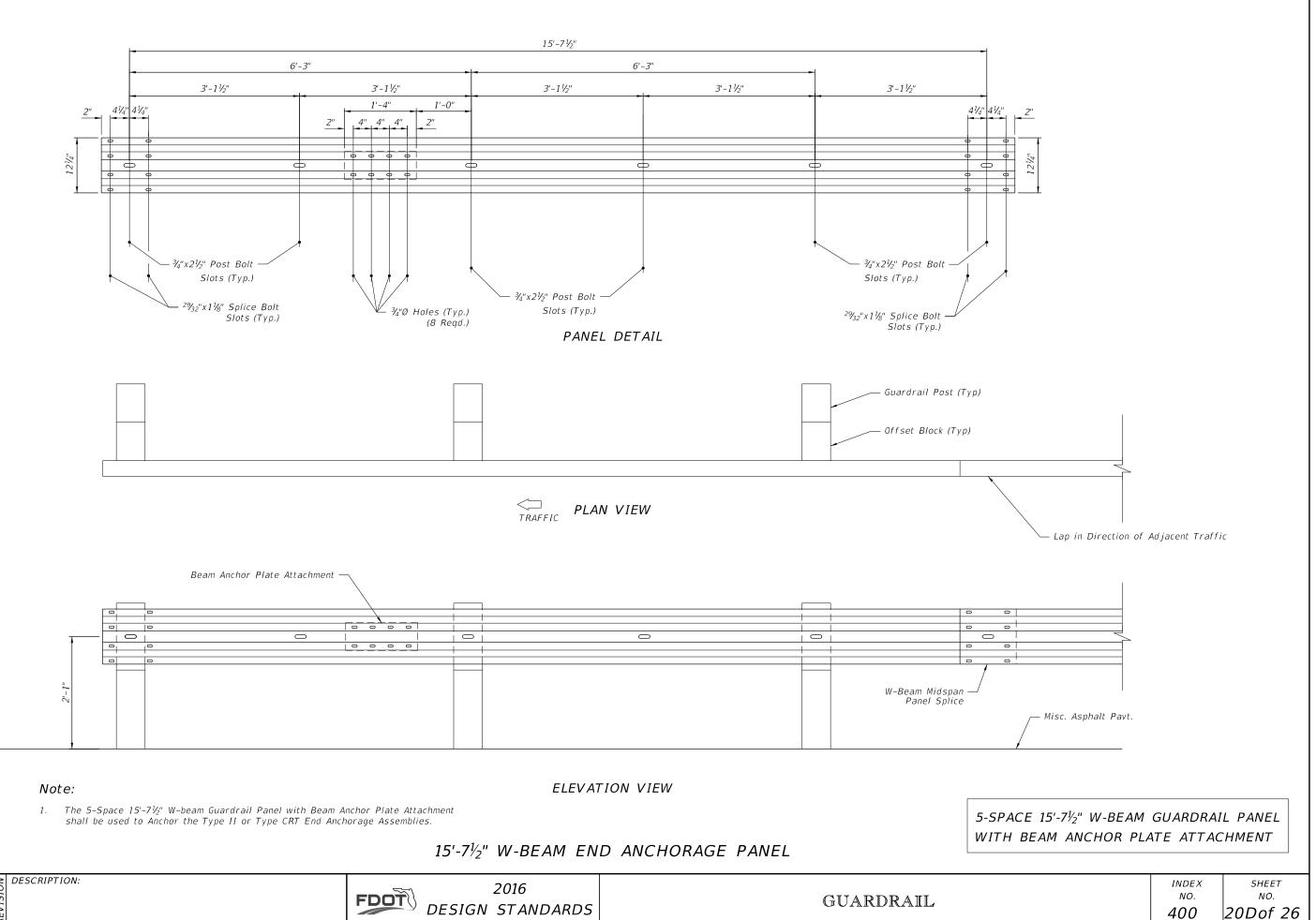


# ASYMMETRICAL W-THRIE BEAM TRANSITION PANEL RWT02 (AASHTO-AGC-ARTBA Report) 10 Gauge

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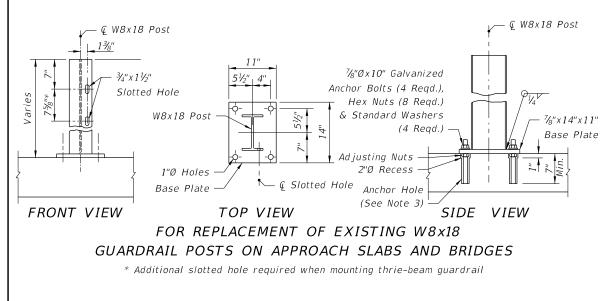






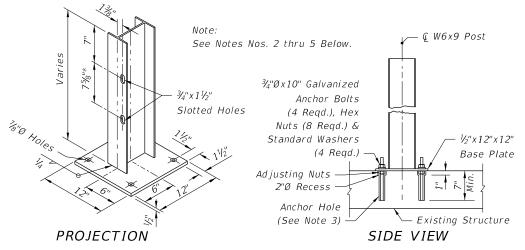
LAST REVISION

01/01/14



### NOTES: (SPECIAL STEEL POST)

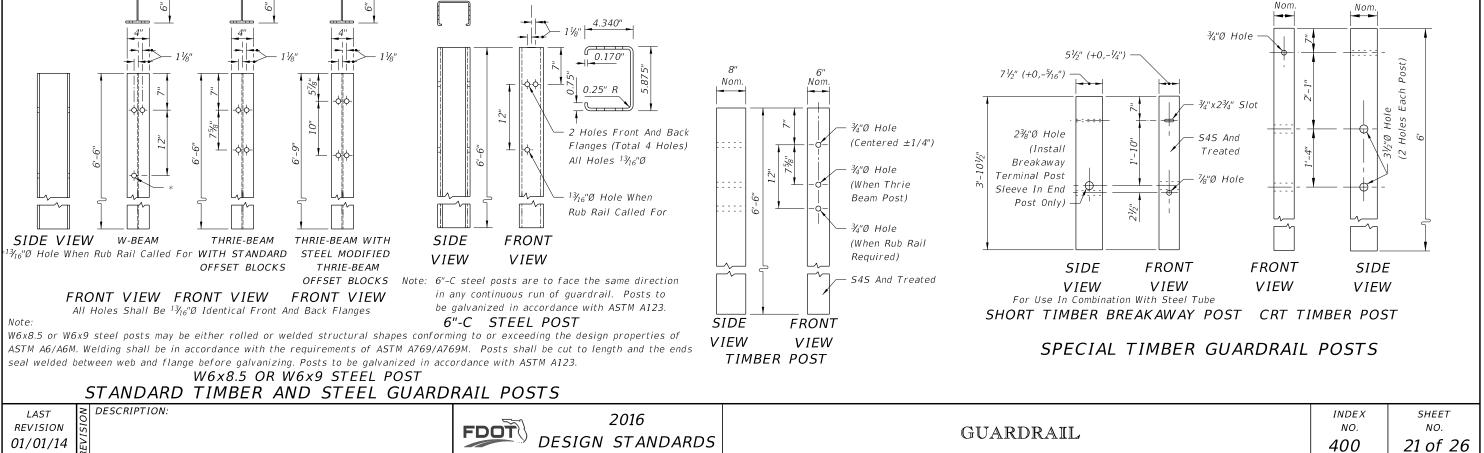
- 1. See Index No. 402 for special steel posts required for construction and repair of guardrail transitions to bridge traffic railing barrier retrofits on existing bridges. See Structures Index Nos. 470 through 476 for steel posts required to construct traffic railing barrier retrofits on existing bridges.
- 2. Either anchor bolts, concrete wedge anchors or approved Adhesive-Bonded Anchors for Structural Applications may be used. Anchor bolts, wedge anchors and adhesive anchors shall have a minimum tensile strength of 60,000 psi and galvanized in accordance with ASTM A153 (stainless steel components may be substituted but components plated in accordance with ASTM B-633 are not acceptable). Adhesive anchor rods shall be equal in diameter to that detailed for anchor bolts. Wedge anchors are to be installed in accordance with the manufacturer's recommendations, assuming 3,000 psi compressive strength for concrete. Wedge anchors shall also meet the following requirements:
- a. tensile load each anchor: approach slabs 14,000 lbs.; other structures 8,000 lbs.
  b. shear load each anchor: approach slabs 15,000 lbs.; other structures 7,800 lbs.

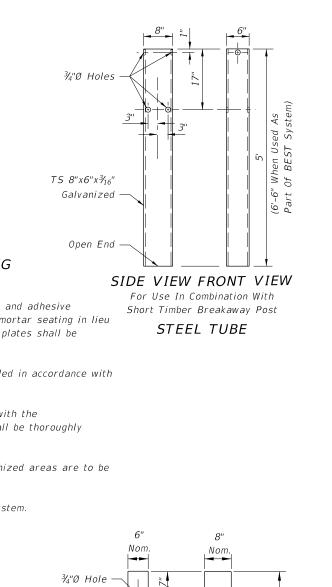


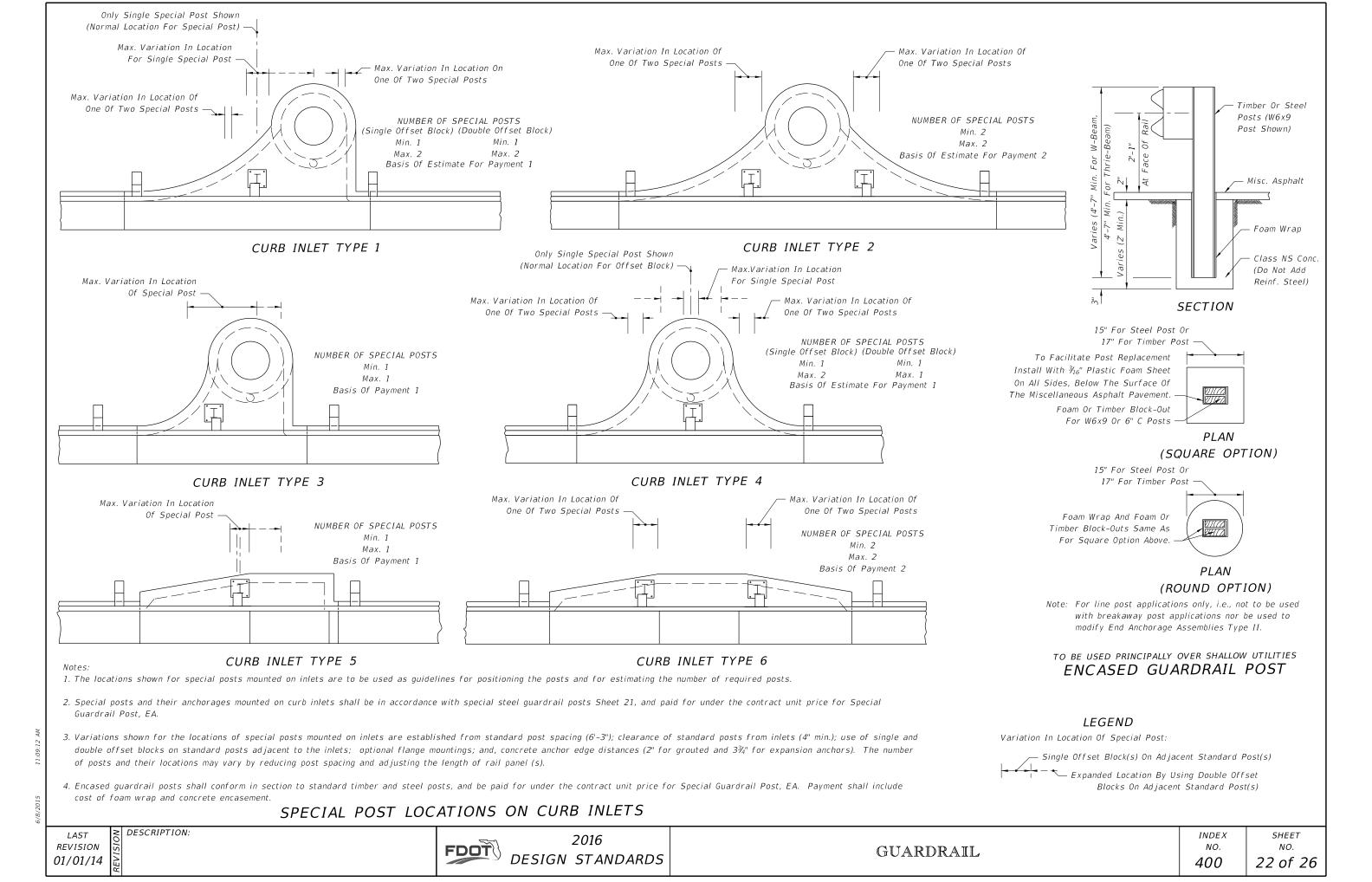
## FOR CONSTRUCTION OF GUARDRAIL WHERE CULVERT, PIER FOOTING OR OTHER STRUCTURE PRECLUDES DRIVEN POST INSTALLATION

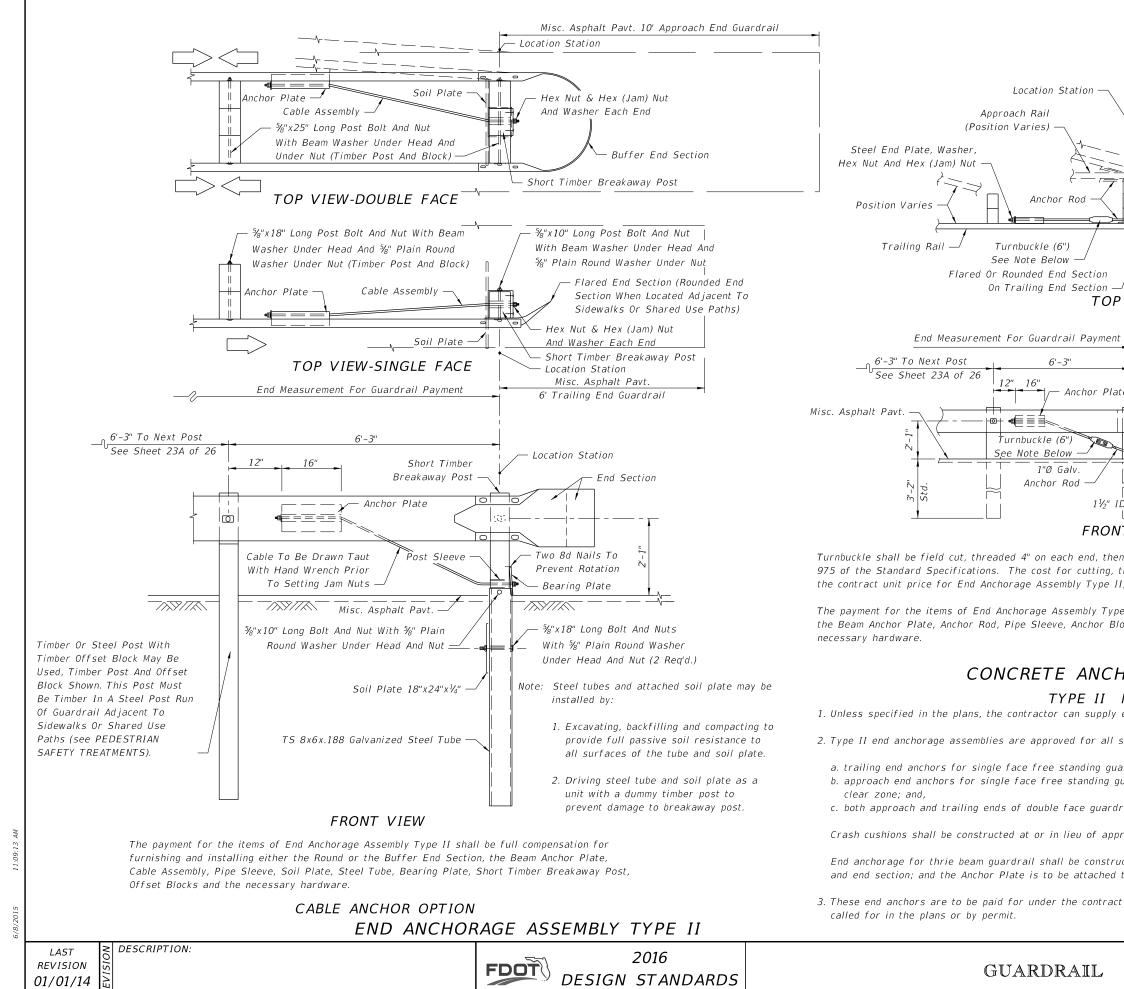
- 3. Posts are to be plumbed by adjusting nuts or mortar seating. Posts installed using anchor bolts and adhesive anchors are to be set with adjusting nuts as detailed, unless the Engineer approves the use of mortar seating in lieu of adjusting nuts. Posts installed using wedge anchors are to be set with mortar seating. Base plates shall be grouted with neat finish.
- 4. Adhesive-Bonded Anchors for Structural Applications shall comply with Section 937 and be installed in accordance with Section 416. Drilled hole diameter shall be in accordance with the manufacturer's instructions.
- 5. Anchor holes and recesses shall be drilled; wedge anchor holes are to be drilled in accordance with the manufacturer's specifications. Encountered reinforcing steel shall be drilled through. Holes shall be thoroughly cleaned when setting bolts and anchors and dry when setting wedge anchors.
- 6. Steel post and base units shall be galvanized in accordance with ASTM A123. Any damaged galvanized areas are to be metalized in accordance with Section 562 of the Standard Specifications.
- 7. Special steel guardrail posts are not to be incorporated into a guardrail approach end anchor system.

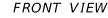
### SPECIAL STEEL GUARDRAIL POSTS











TOP VIEW

Turnbuckle shall be field cut, threaded 4" on each end, then cleaned and metalized in accordance with Sections 562 and 975 of the Standard Specifications. The cost for cutting, threading, metalizing and the turnbuckle shall be included in the contract unit price for End Anchorage Assembly Type II, EA.

The payment for the items of End Anchorage Assembly Type II shall be full compensation for furnishing and installing the Beam Anchor Plate, Anchor Rod, Pipe Sleeve, Anchor Block, either Flared, Rounded or Buffer End Section, and the

# CONCRETE ANCHOR BLOCK OPTION

6'-3"

Anchor Plate

### TYPE II NOTES

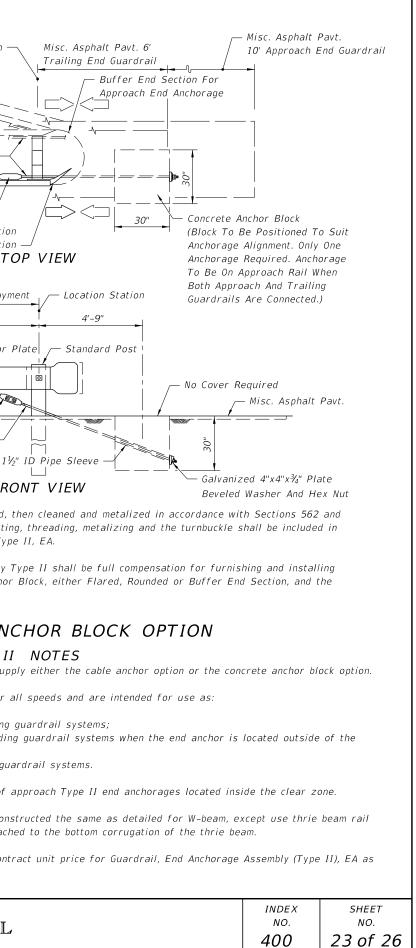
- 1. Unless specified in the plans, the contractor can supply either the cable anchor option or the concrete anchor block option.
- 2. Type II end anchorage assemblies are approved for all speeds and are intended for use as:
- a. trailing end anchors for single face free standing guardrail systems;
- b. approach end anchors for single face free standing guardrail systems when the end anchor is located outside of the
- c. both approach and trailing ends of double face guardrail systems.

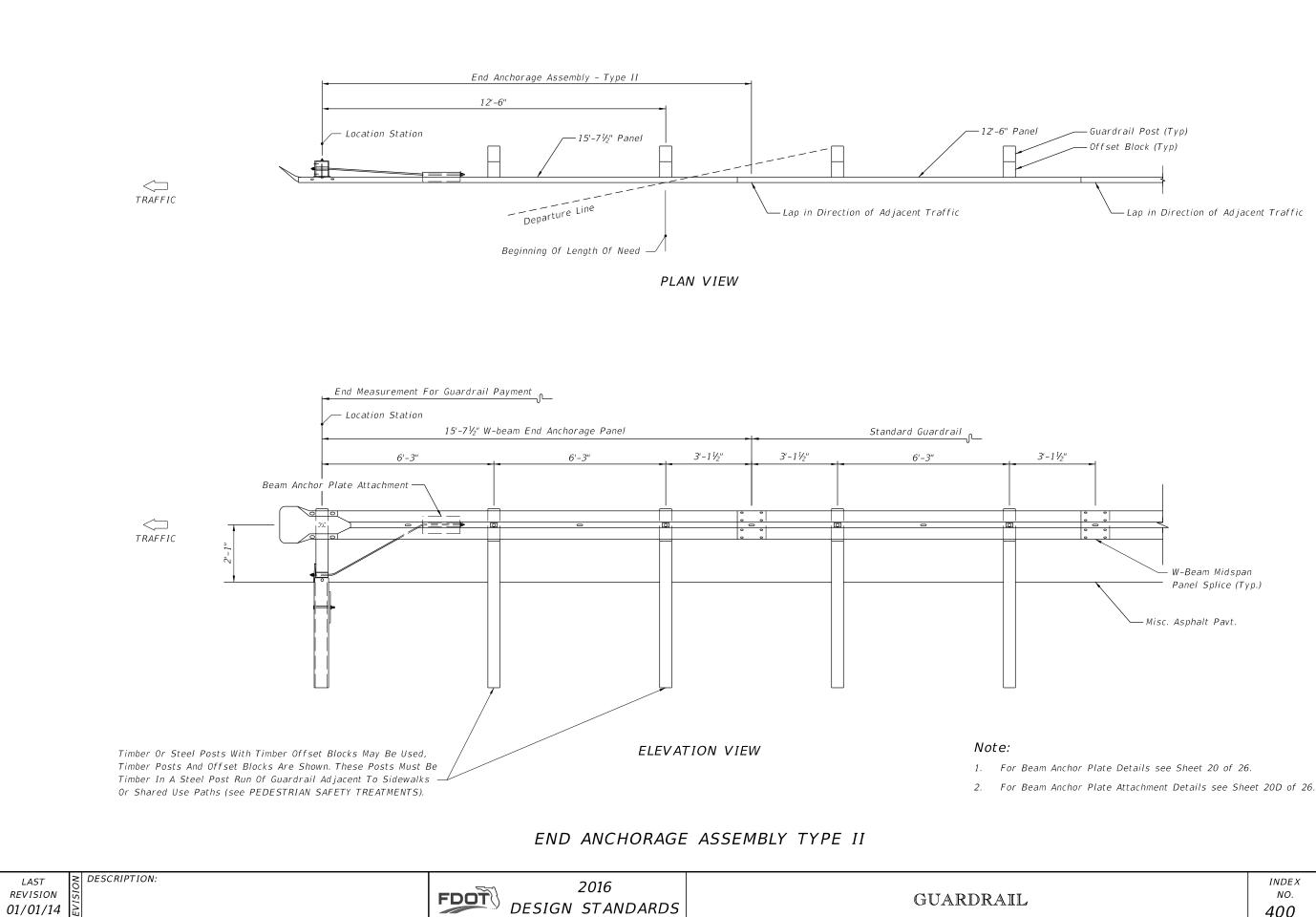
Crash cushions shall be constructed at or in lieu of approach Type II end anchorages located inside the clear zone.

End anchorage for thrie beam guardrail shall be constructed the same as detailed for W-beam, except use thrie beam rail and end section; and the Anchor Plate is to be attached to the bottom corrugation of the thrie beam.

3. These end anchors are to be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Type II), EA as

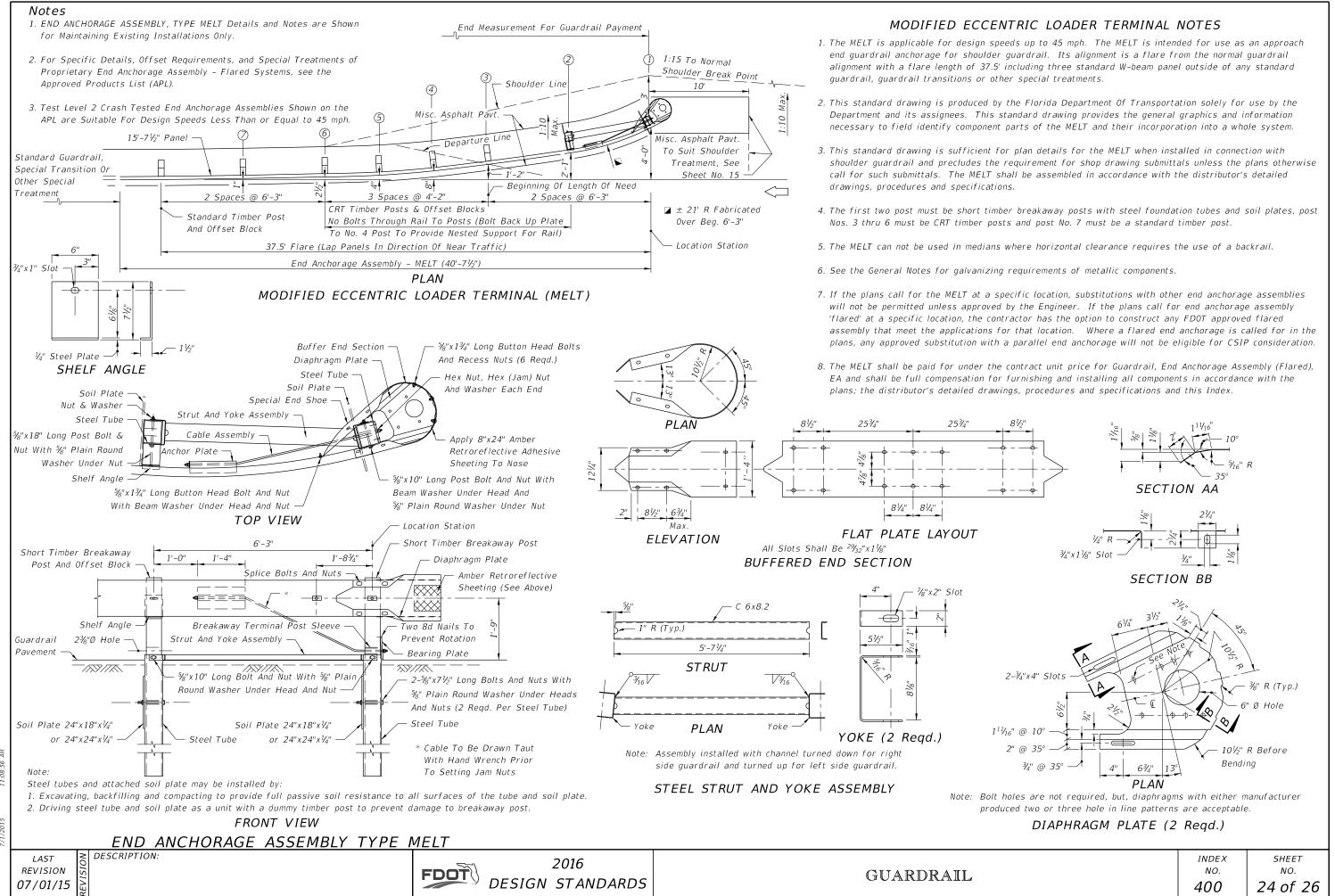
GUARDRAIL

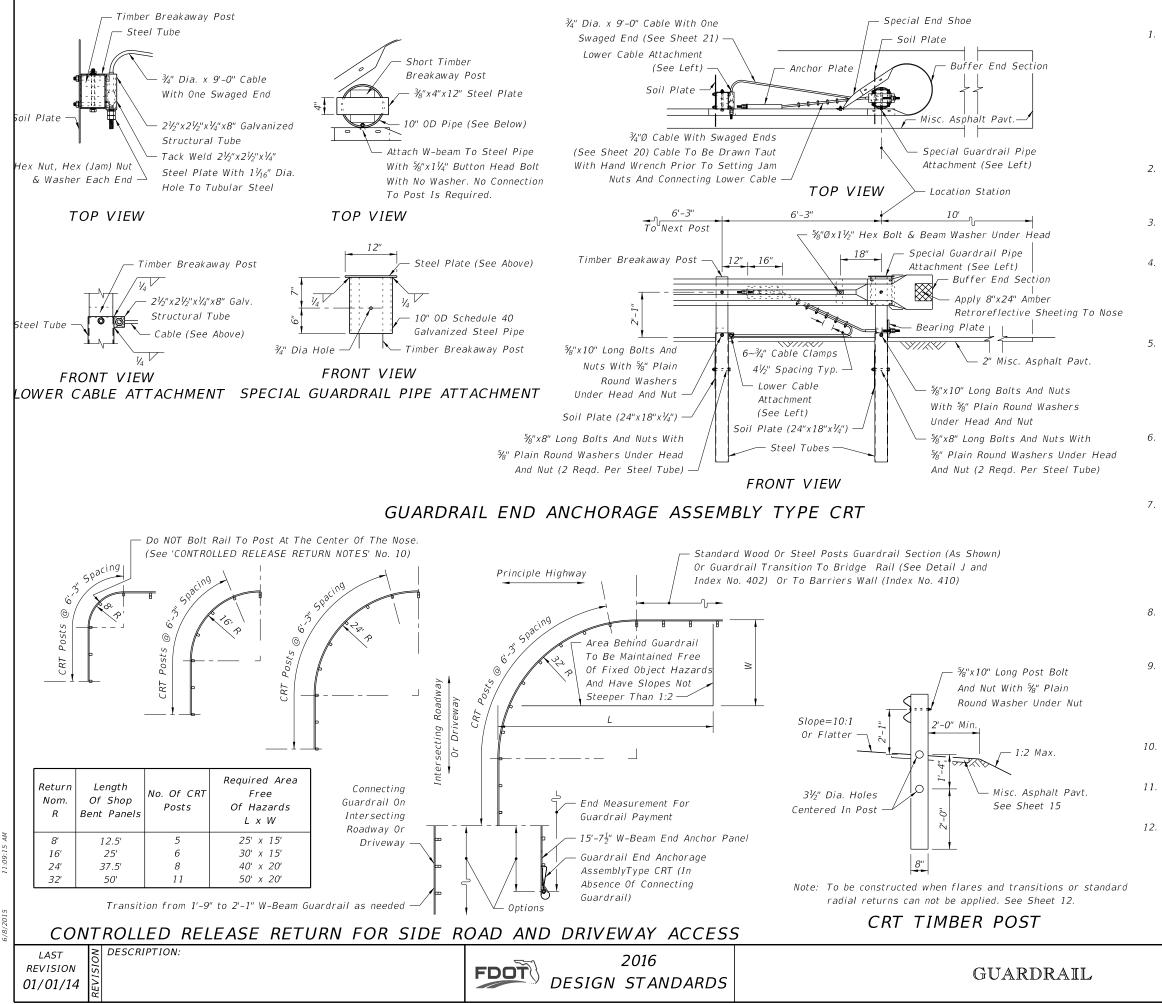




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### CONTROLLED RELEASE RETURN NOTES

 Controlled release returns are intended for use (a) in openings in continuous guardrail for driveway and side road access when flares and transitions or standard radial returns can not be applied (Sheet 12); and, (b) for shielding the ends of bridge traffic rails and barrier walls where the driveway and side road access is in close proximity to the structure and space does not permit the proper use of approved flared and parallel types of Guardrail End Anchorage Assemblies.

2. Controlled release returns are not intended as a substitute or replacement for the appropriate use of approved vehicle impact attenuators.

3. Controlled release returns with either 8', 16' or 24' radii are designed for highway speeds of 60 mph or less.

4. The controlled release returns shown are designed as full returns based on an intersection angle of 90°. The return can be terminated with the Guardrail End Anchorage Assembly Type CRT or connected to standard guardrail as shown or as otherwise detailed in the plans.

5. The Guardrail End Anchorage Assembly Type CRT is to be used only for the controlled release returns with 8', 16', 24' and 32' radii as shown; the assembly is not to be used in any tangent rail or flared rail applications. Other types of end anchorage assemblies are not to be used in the controlled release returns.

6. The area immediately behind the control release return shall have slopes not steeper than 1:2 and be maintained free of fixed objects in accordance with the area limits tabulated in the plan below.

7. The surface approaching the controlled release return shall have a transverse slope not exceeding 1:10. The effective width of the transverse surface is to be based on standard vehicle departure, return radii and preceding shielding; the width (beyond shoulder) shall be not greater than the corresponding 15' and 20', 'W' values tabulated below.

8. The curved guardrail portion of the controlled release return shall be full section shop bent panels (12.5' or 25' panels).

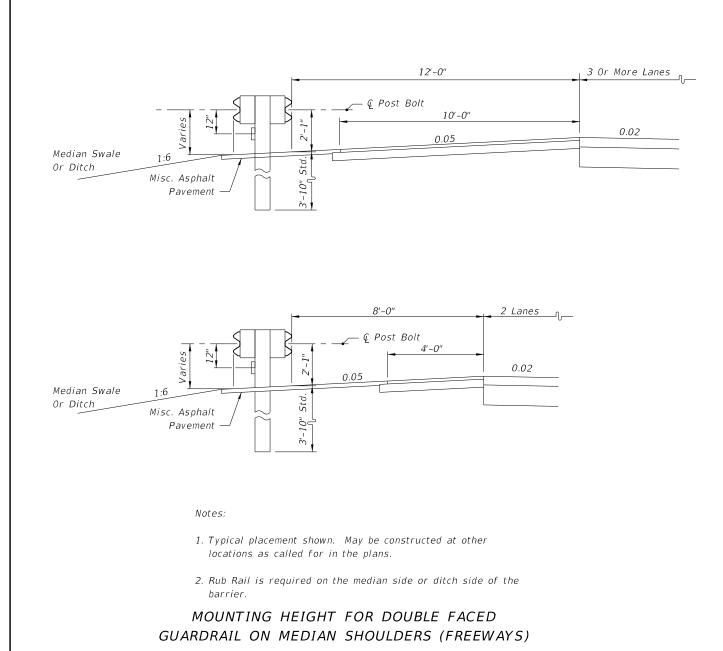
9. Washers are not to be used between the guardrail beam and the head of the button head post bolts at any controlled release terminal (CRT) post or at any Guardrail End Anchorage Assembly Type CRT breakaway timber post.

10. The guardrail beam of the 8' radius return is not bolted to the center control release post.

11. See the General Notes for galvanizing requirements of metallic components.

12. Controlled release return systems shall be paid for under the contract unit prices for Guardrail (Roadway), LF, Guardrail (Shop-bent Panels), LF, and Guardrail, End Anchorage Assembly (Type CRT), EA as called for in the plans or by permit and shall be full compensation for furnishing and installing all components in accordance with the plans and with this index. CRT posts are included in the cost for guardrail.

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### LATERAL PLACEMENT ON SLOPES (FROM EDGE OF NEAR TRAFFIC LANE)<sup>1</sup> Guardrail Standard Guardrail Slope Not Guardrail<sup>2</sup> With Rub Rail<sup>3</sup> Recommended 1:6 to 16' 17' to 22' 23' to 45' 21' to 24' 1:7 to 20' 25' to 45' to 25' 1:8 26' to 45' 1:9 to 26' 27' to 45' 1:10 to 27' 28' to 45'

Lateral Offset



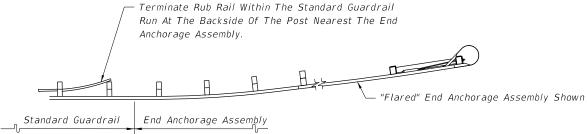
Shoulder

Traffic

Lane

\* C6x8.2, Plates And Fasteners or Bent Plate And Fasteners In Accordance With Standards RLR01 And RER01 Of AASHTO-AGC-ARTBA "A Guide To Standardized Highway Barrier Hardware"

# GUARDRAIL ON SLOPES



RUB RAIL TERMINATION

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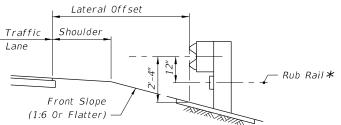
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2016 DESIGN STANDARDS

GUARDRAIL

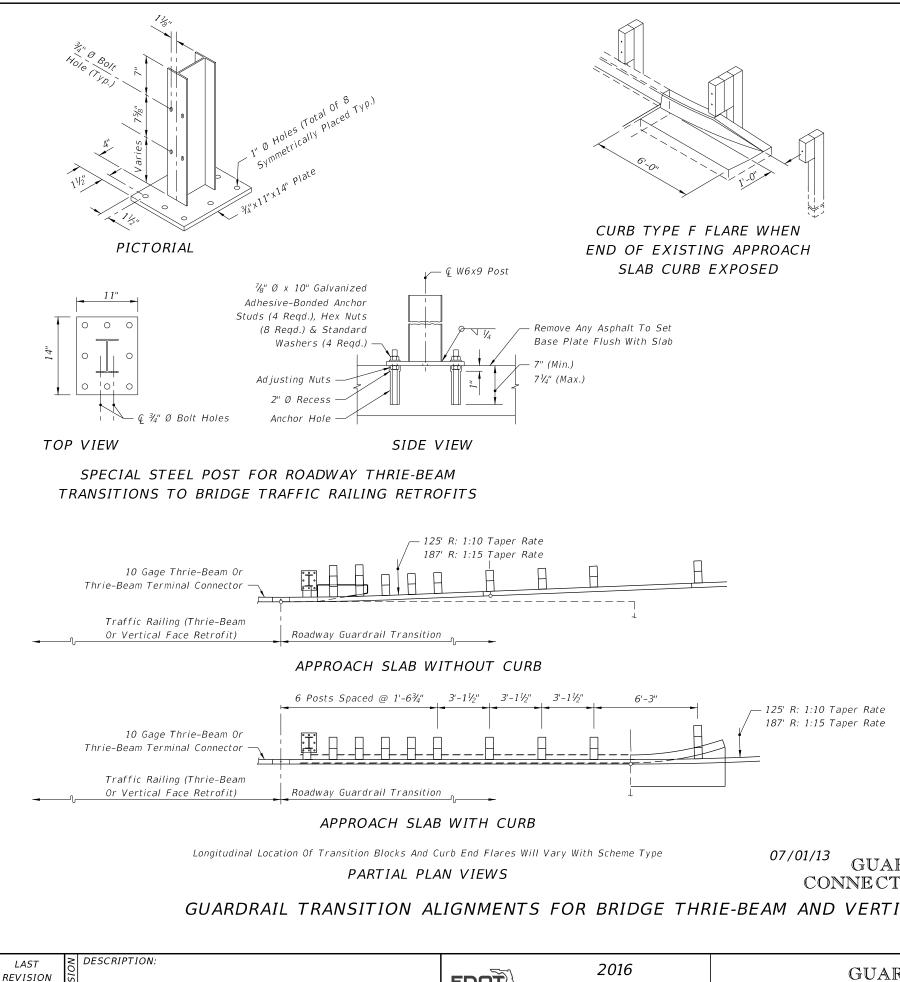
Notes:

- 1. For shoulders less than 12' in width the tabulated values will be reduced by the difference between 12' and the shoulder width. Placement of guardrail on front slopes steeper than 1:6 not recommended.
- 2. Standard guardrail; 2'-1" to Q post bolt. Rub Rail is required on the median side when double face guardrail is used.
- 3. Guardrail with Rub Rail; 2'-4" to Q post bolt.



### GUARDRAIL ON OUTSIDE SLOPES

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FDOT

- with safety shaped traffic railing.
- (IDS-470 & IDS-480).
- provided on this index, refer to Index No. 400.

# NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- railings, and (b) depict the typical alignments of the approach transitions.
- curb blunt ends are not in place.
- hot-dip zinc coated in accordance with Section 536 of the Specifications.

- particular scheme. The associated pictorial views show the variations.
- notations on Sheets 12 through 15 and the flag notation on Sheet 23.

# DESIGN NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT

DESIGN STANDARDS

GUARDRAIL TRANSITIONS A CONNECTIONS FOR EXISTING BI

07/01/13

### GENERAL NOTES

1. This index provides thrie-beam transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 23 apply to bridges with retrofitted traffic railings, (Sheet 23 shows the trailing end guardrail connections). Sheet 24 applies to bridges

2. The schemes identified by Arabic numerals in this index are complementary to the bridge traffic railing barrier retrofit schemes with like numeral identification in Index Nos. 470, 471 through 476, 480 through 483. The schemes in this index identified by Roman numerals are complementary to bridge safety shaped traffic railing barrier where determined to be in accordance with applications of criteria specified in the Instructions for Design Standards

3. For guardrail applications and details of related hardware and accessories that are not

1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic

2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to

3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated using ASTM A36 or ASTM A709 Grade 36 steel. Welding shall conform to ANSI/AASHTO/AWS D1.5. The assembly shall be

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each

5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see

6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

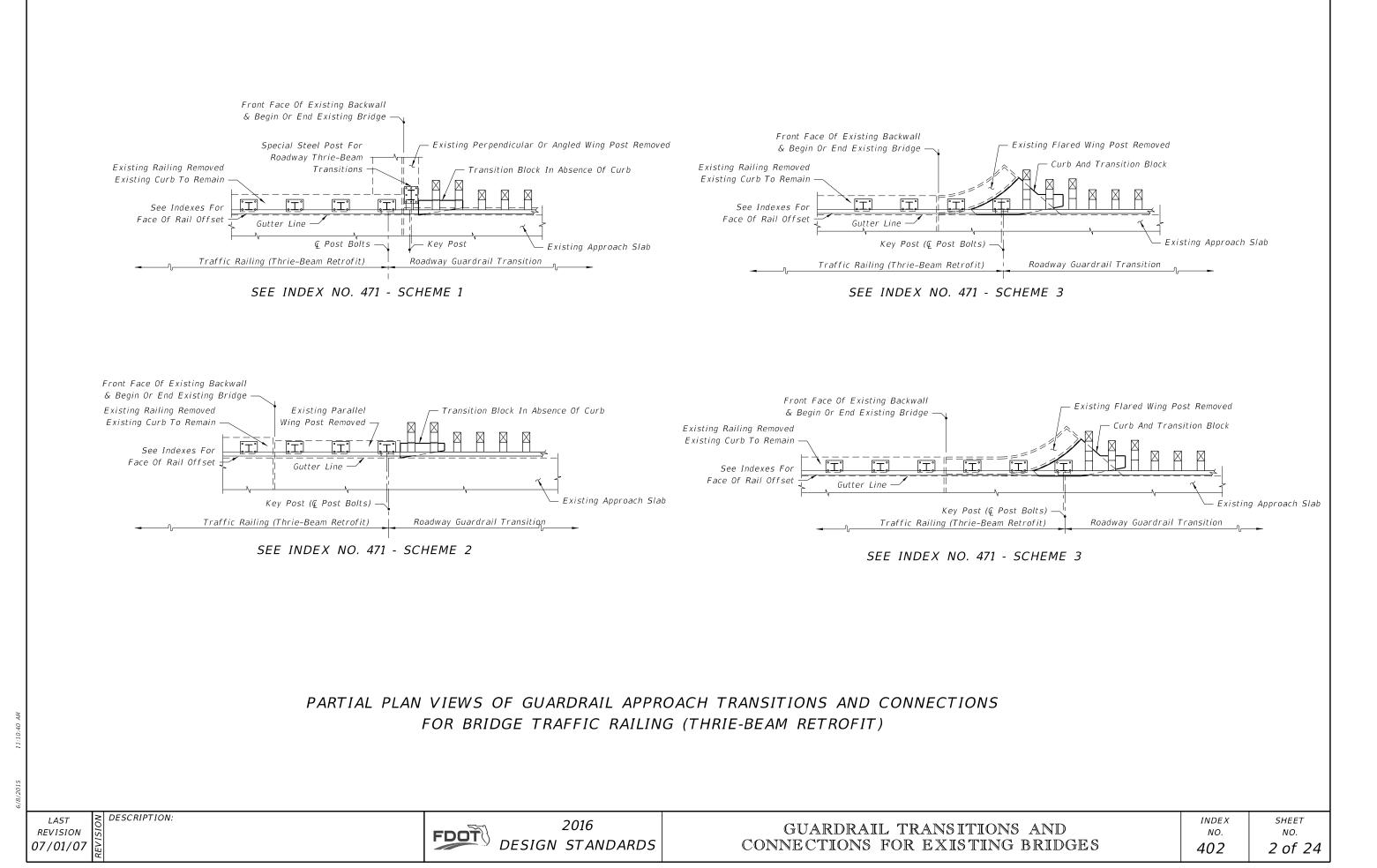
1. For selection of an appropriate transition scheme, see the Instructions for Design Standards (IDS-470 & IDS-480) for instructions to the Structures and Roadway engineers.

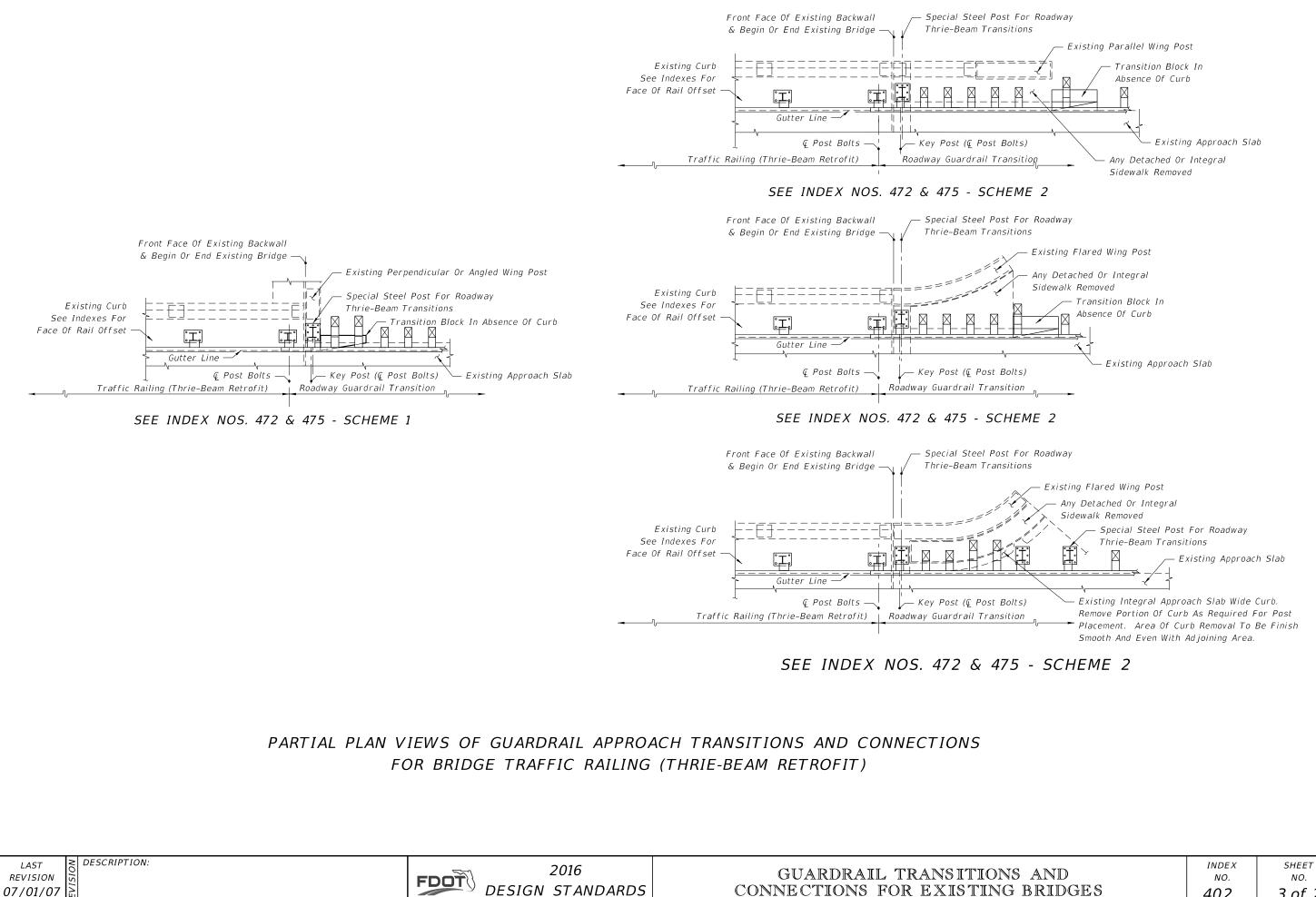
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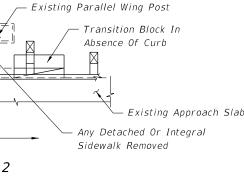
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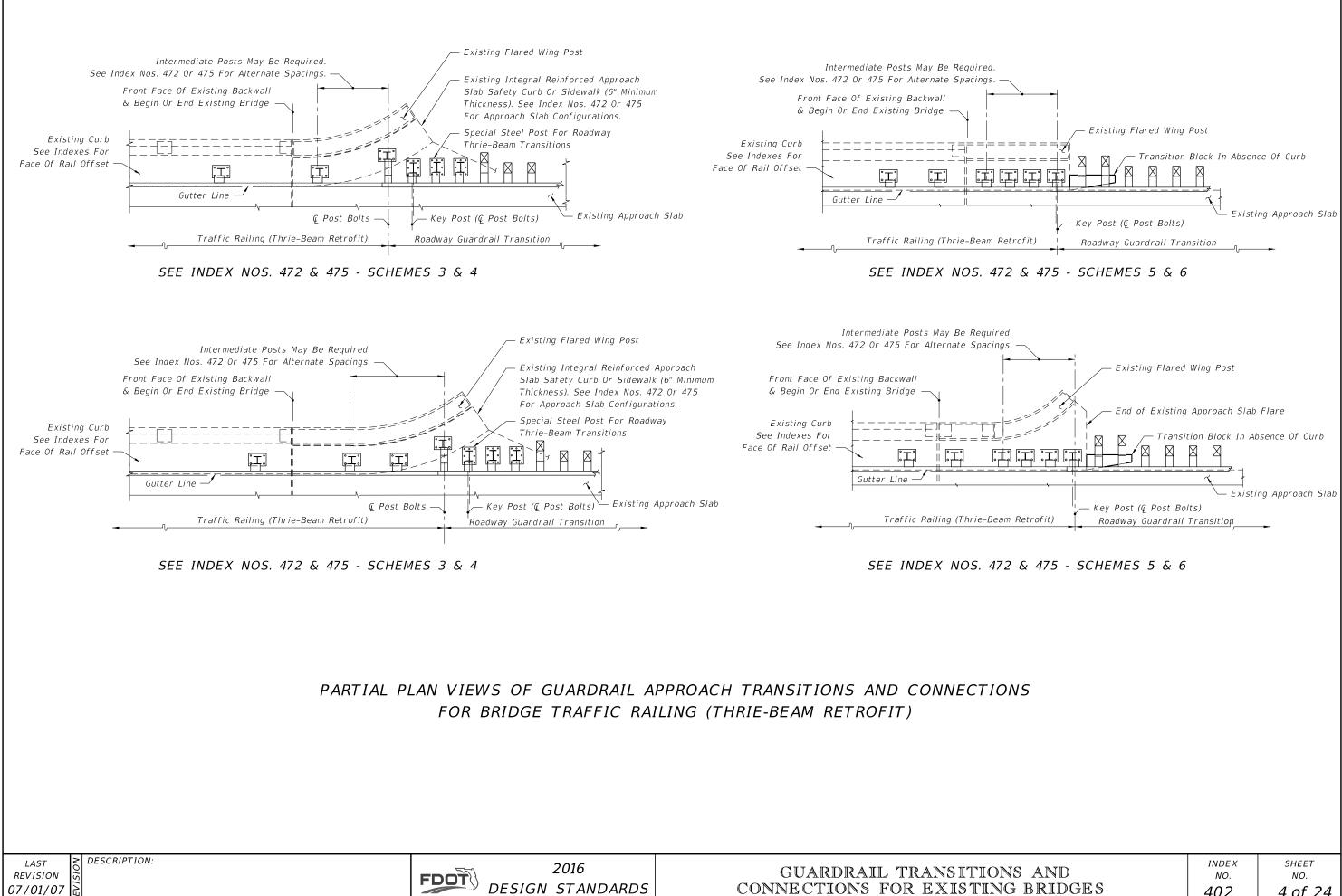
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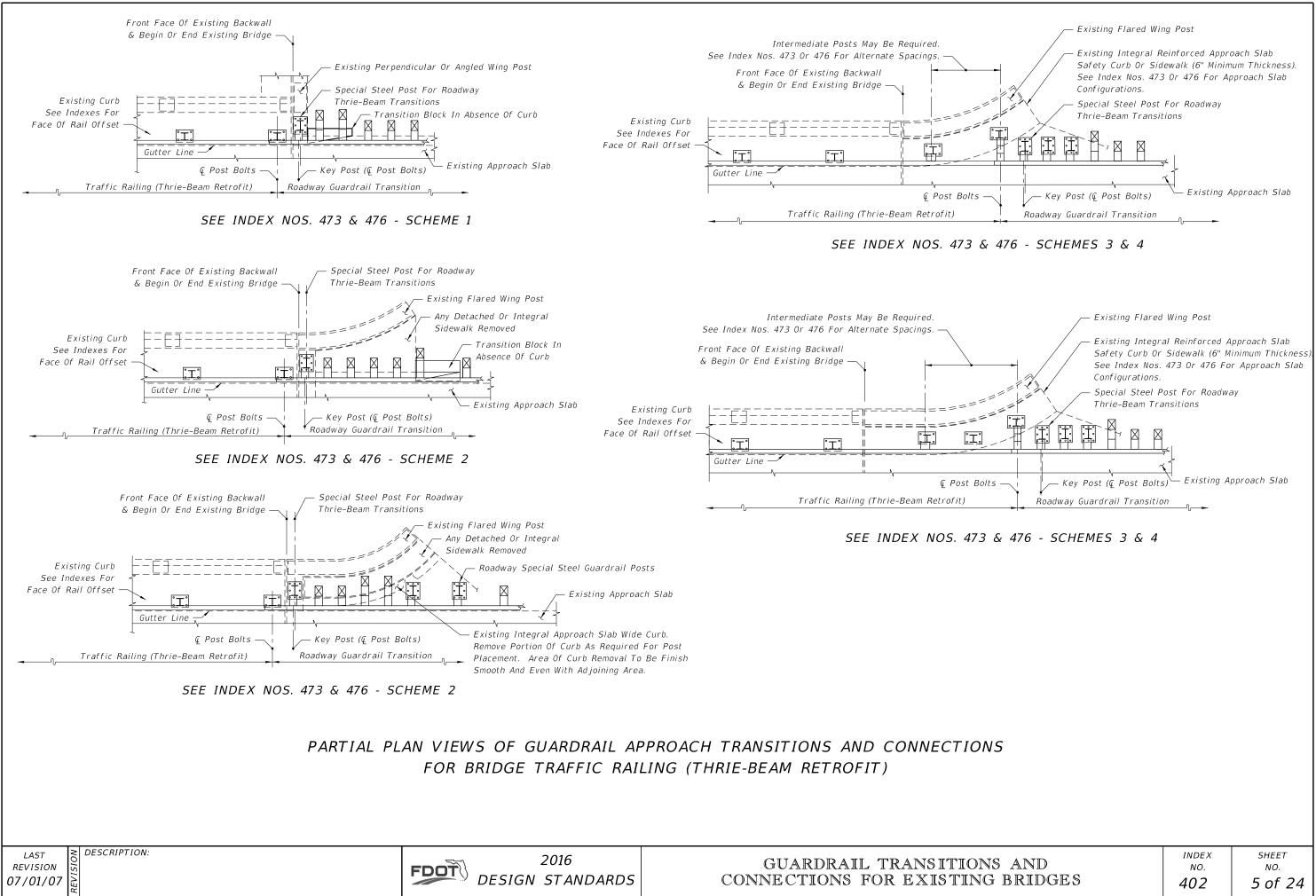




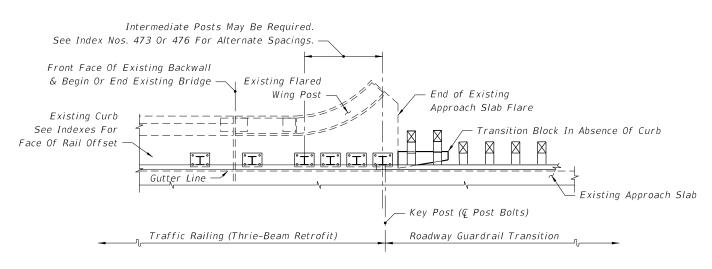
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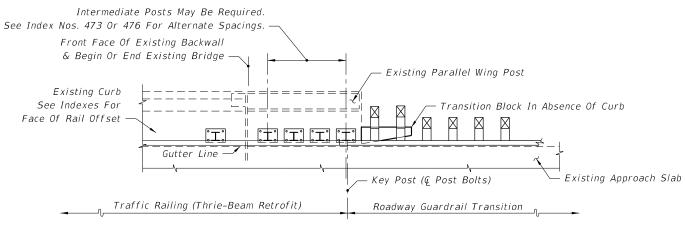
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SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6



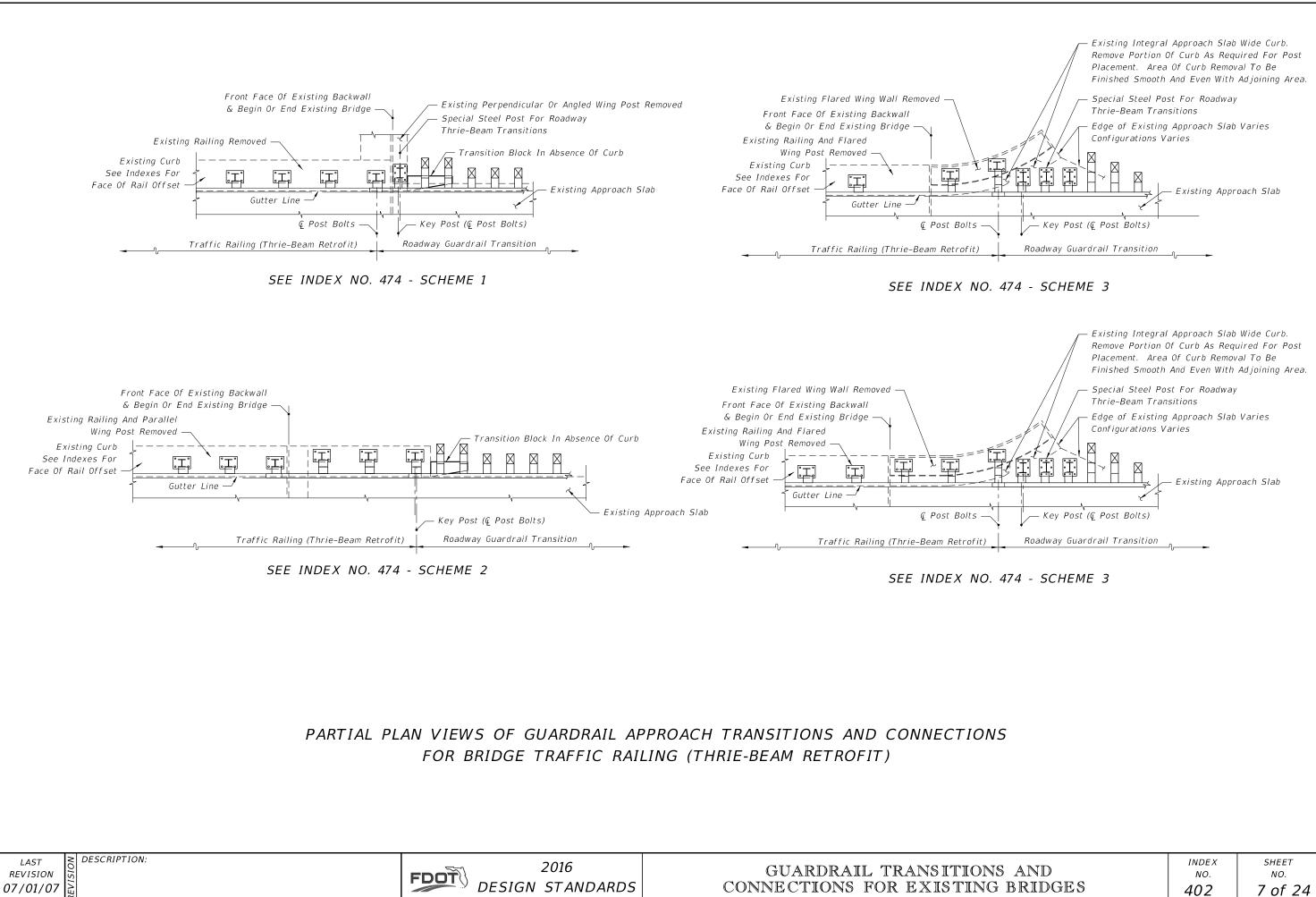
SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

## PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

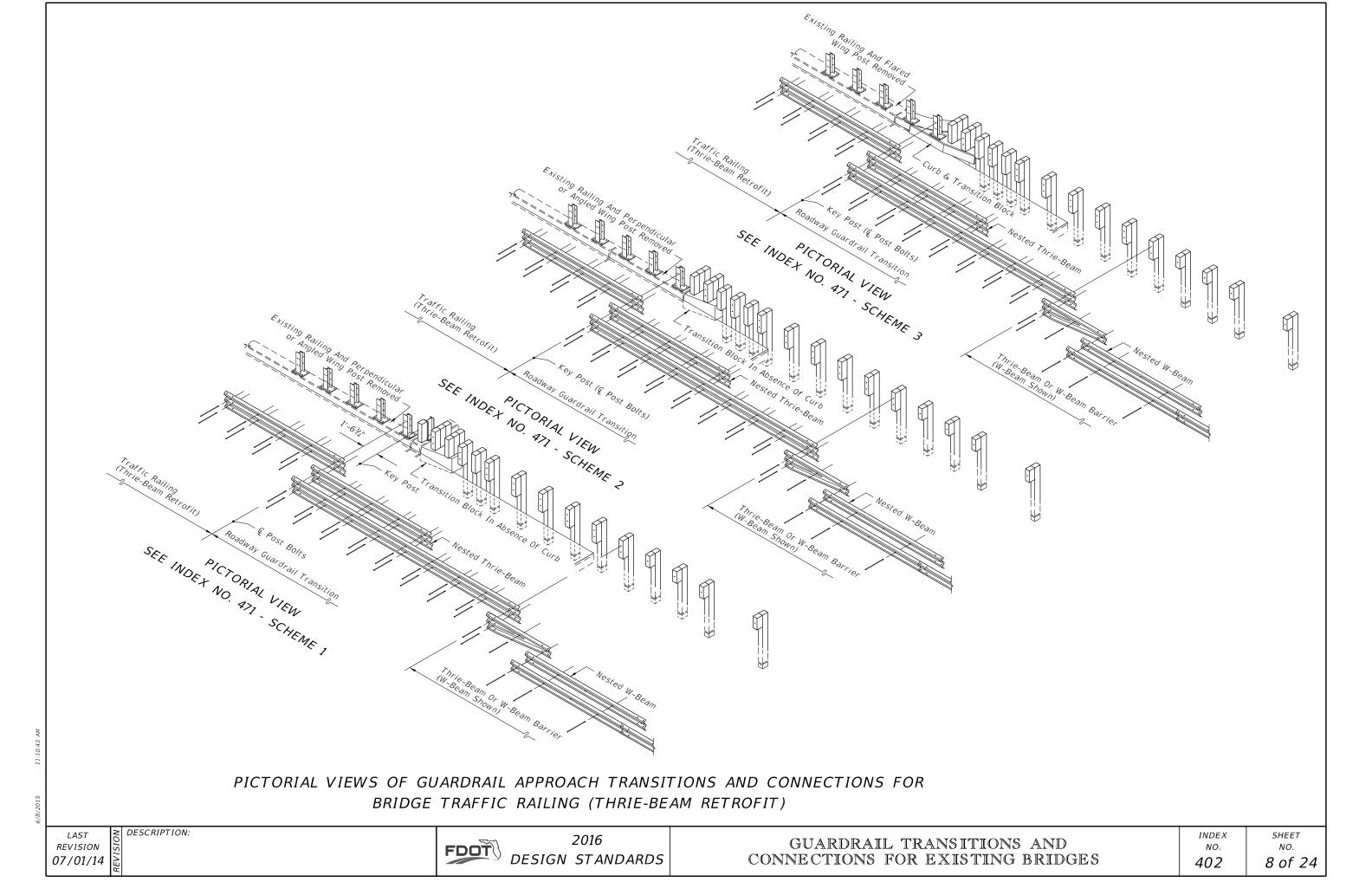
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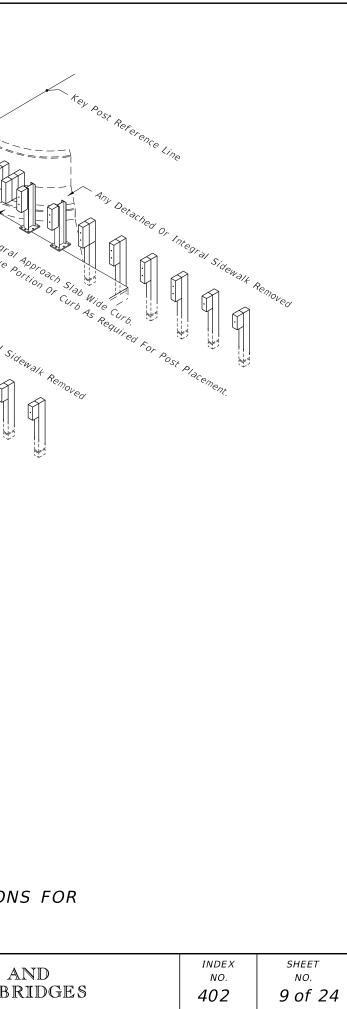
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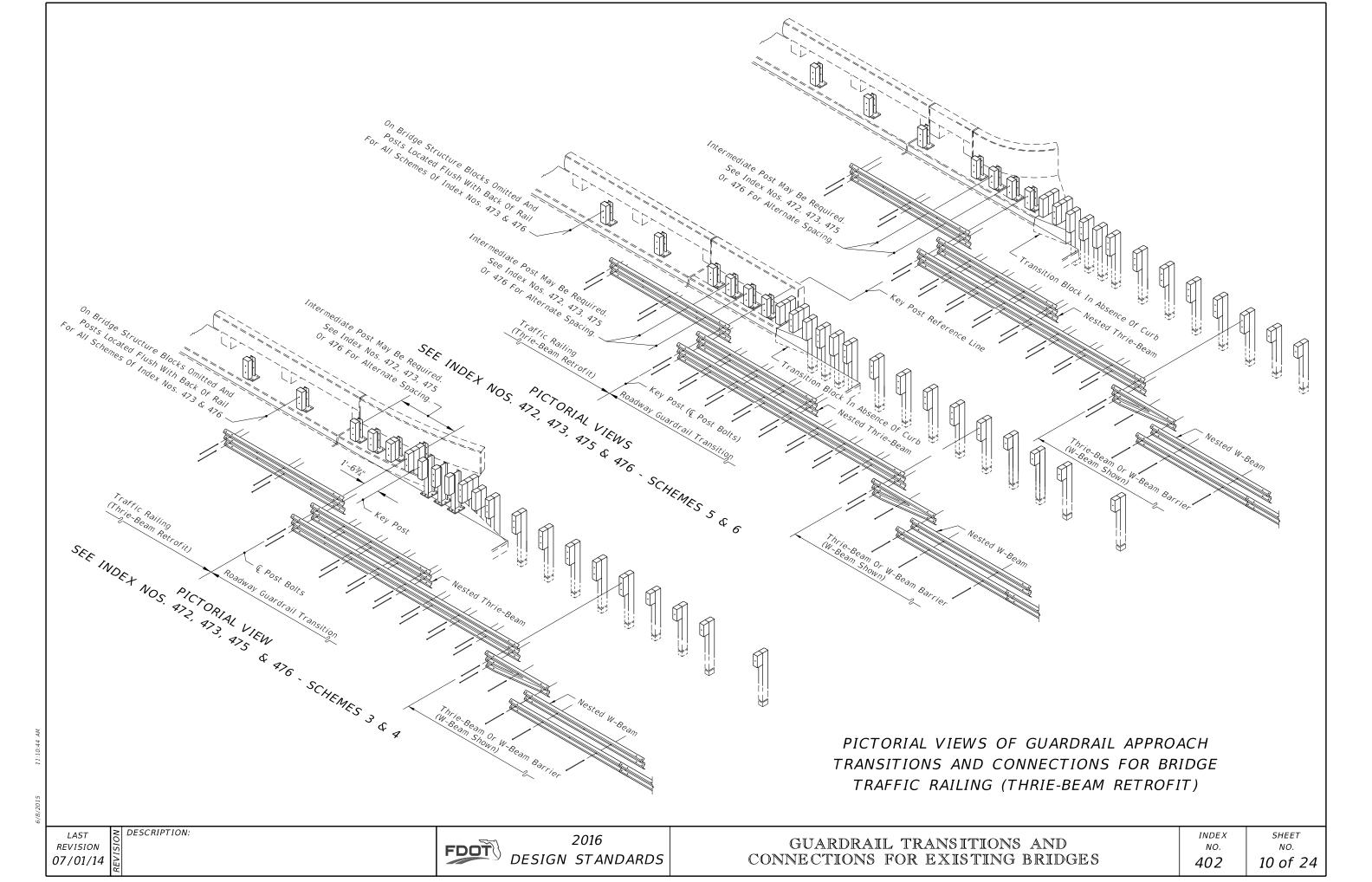
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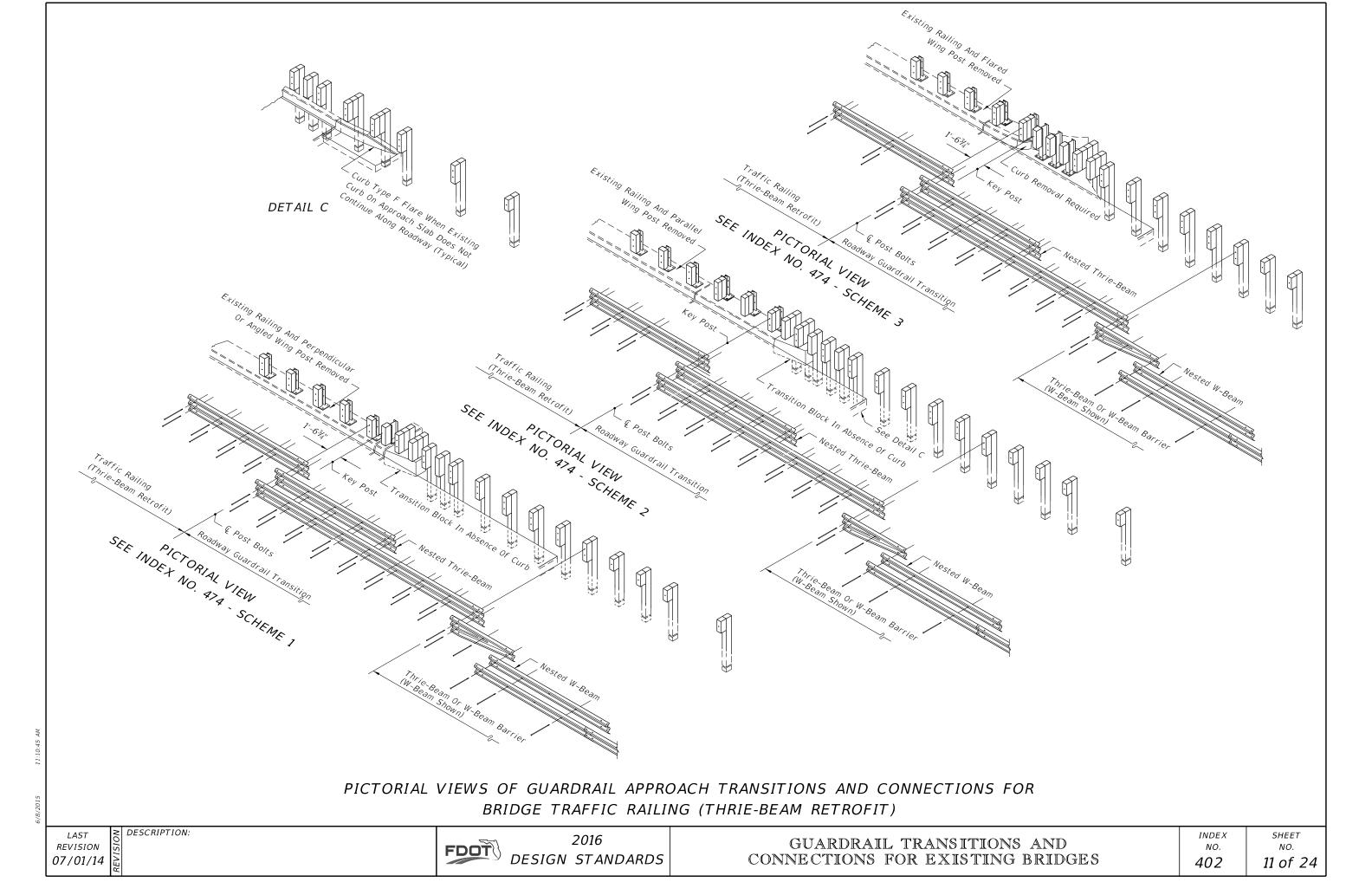


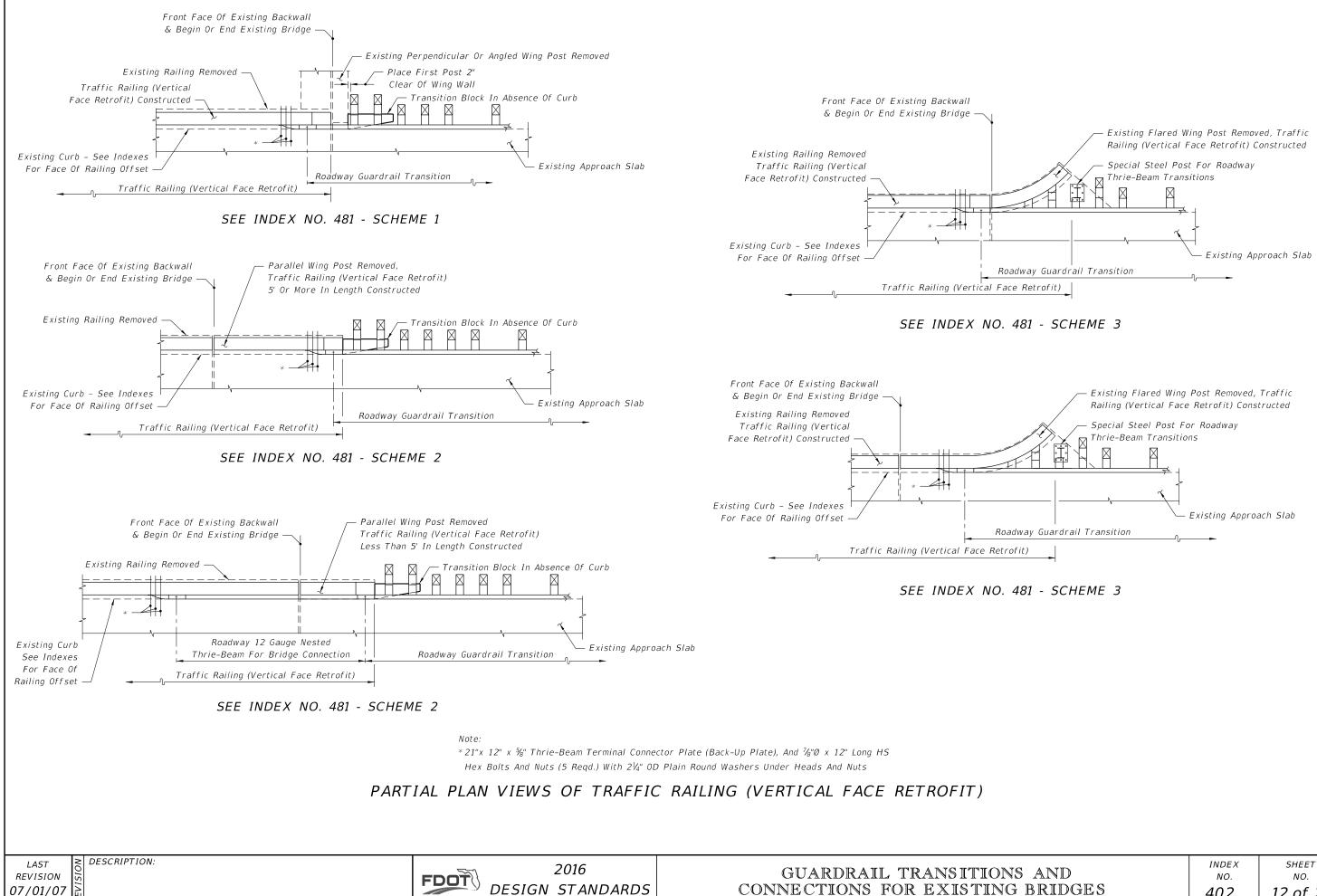
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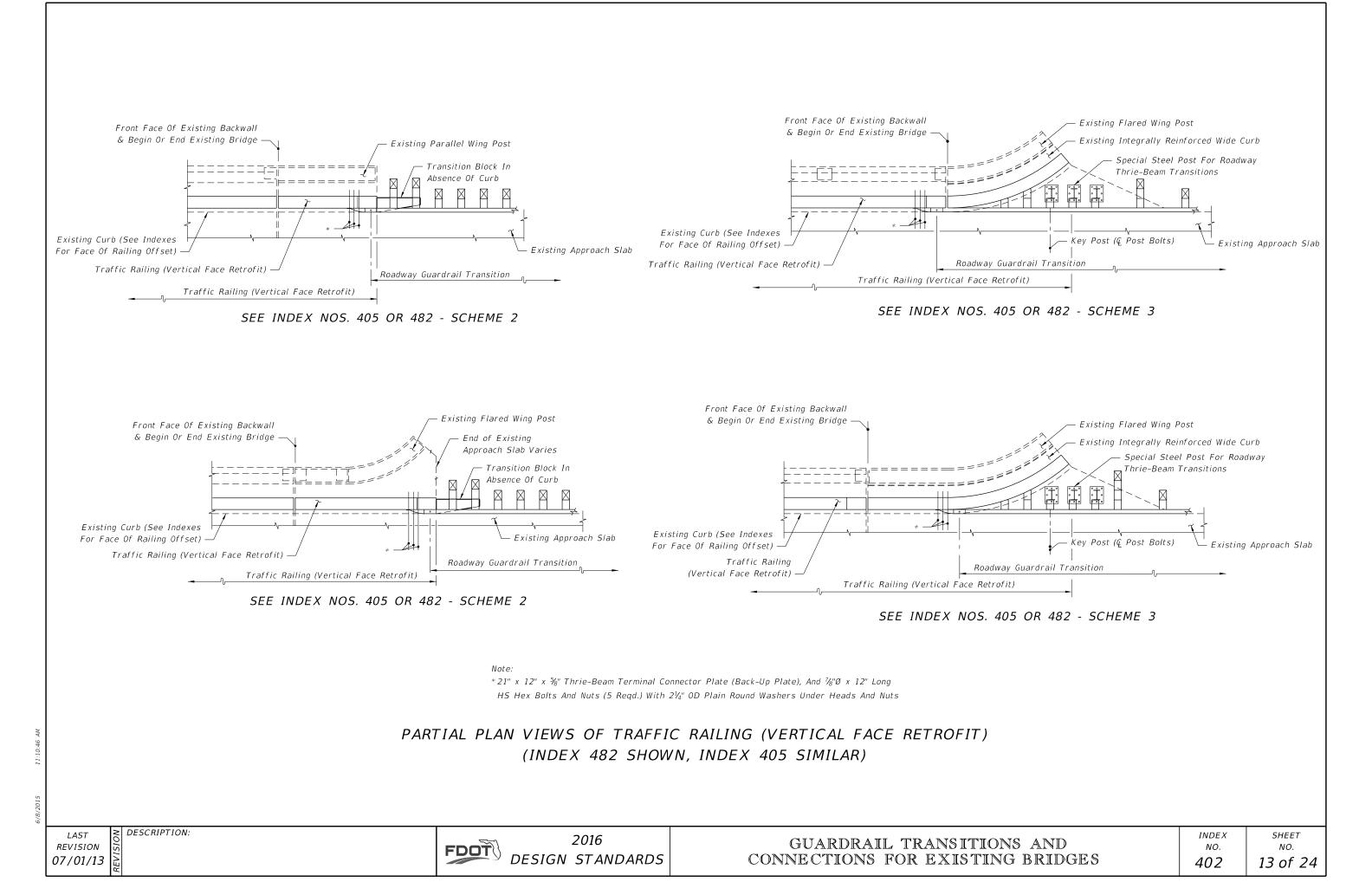


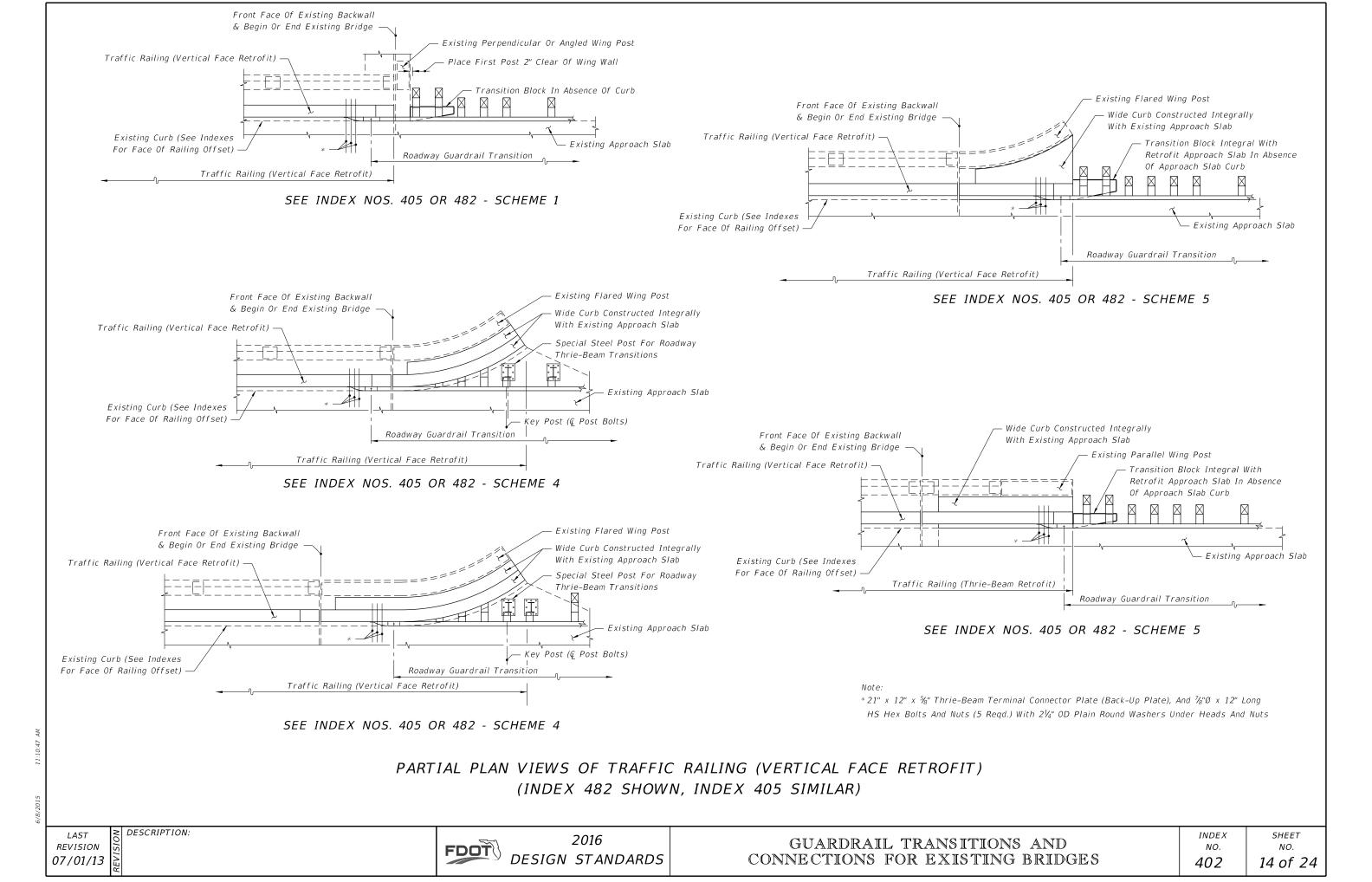


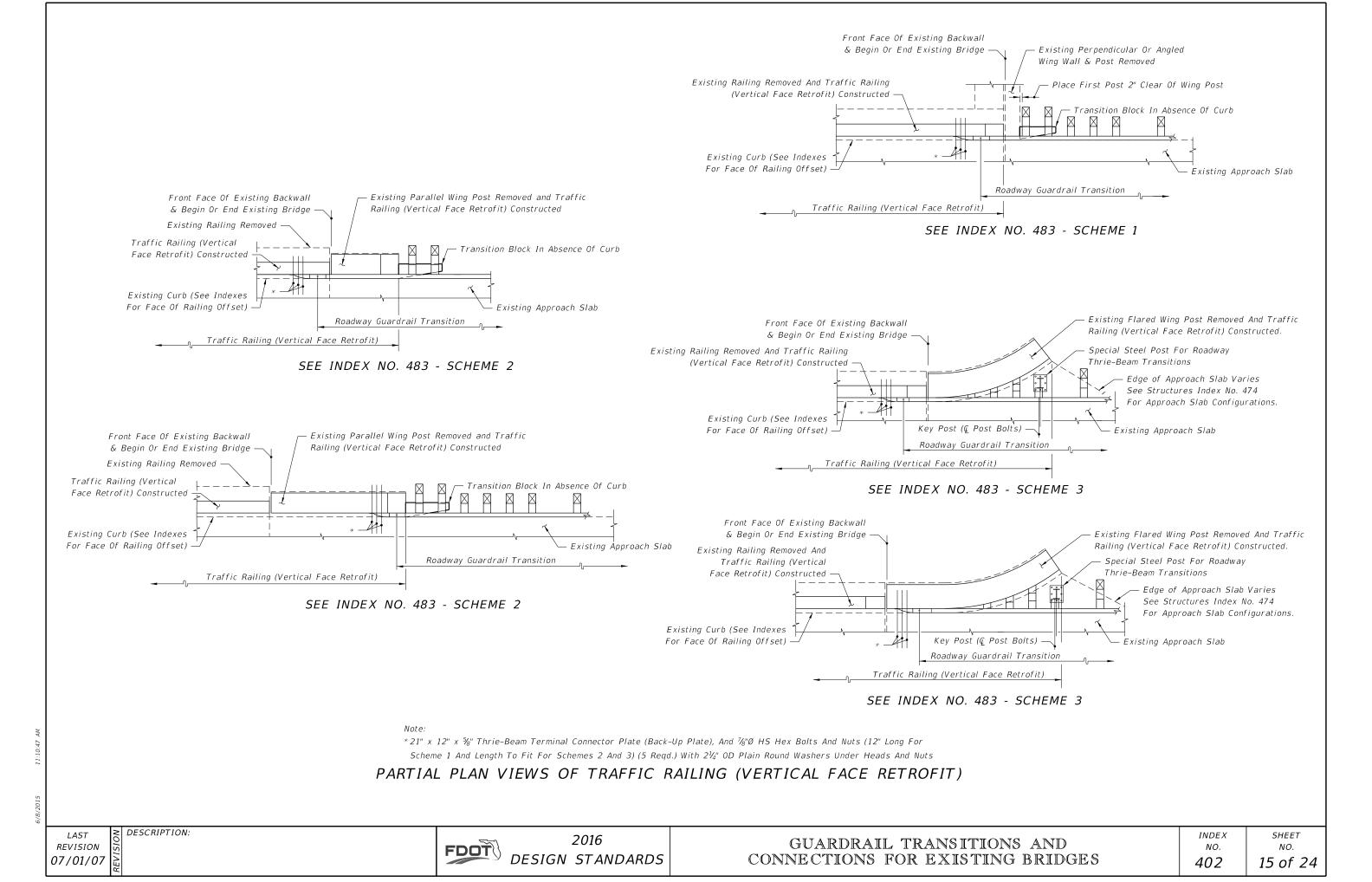


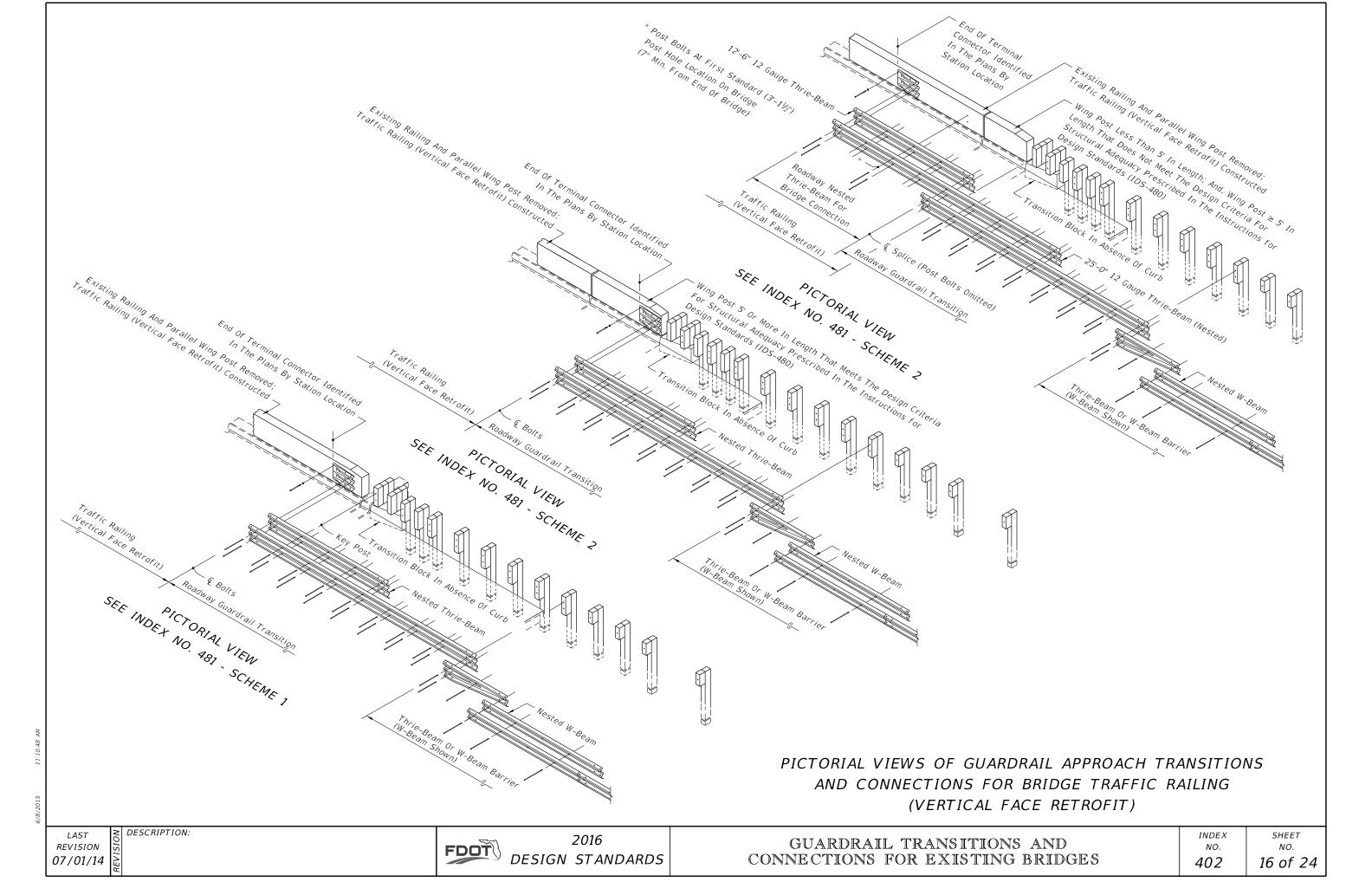
— Existing Flared Wing Post Removed, Traffic Railing (Vertical Face Retrofit) Constructed
Special Steel Post For Roadway
Existing Approach Slab
Guardrail Transition

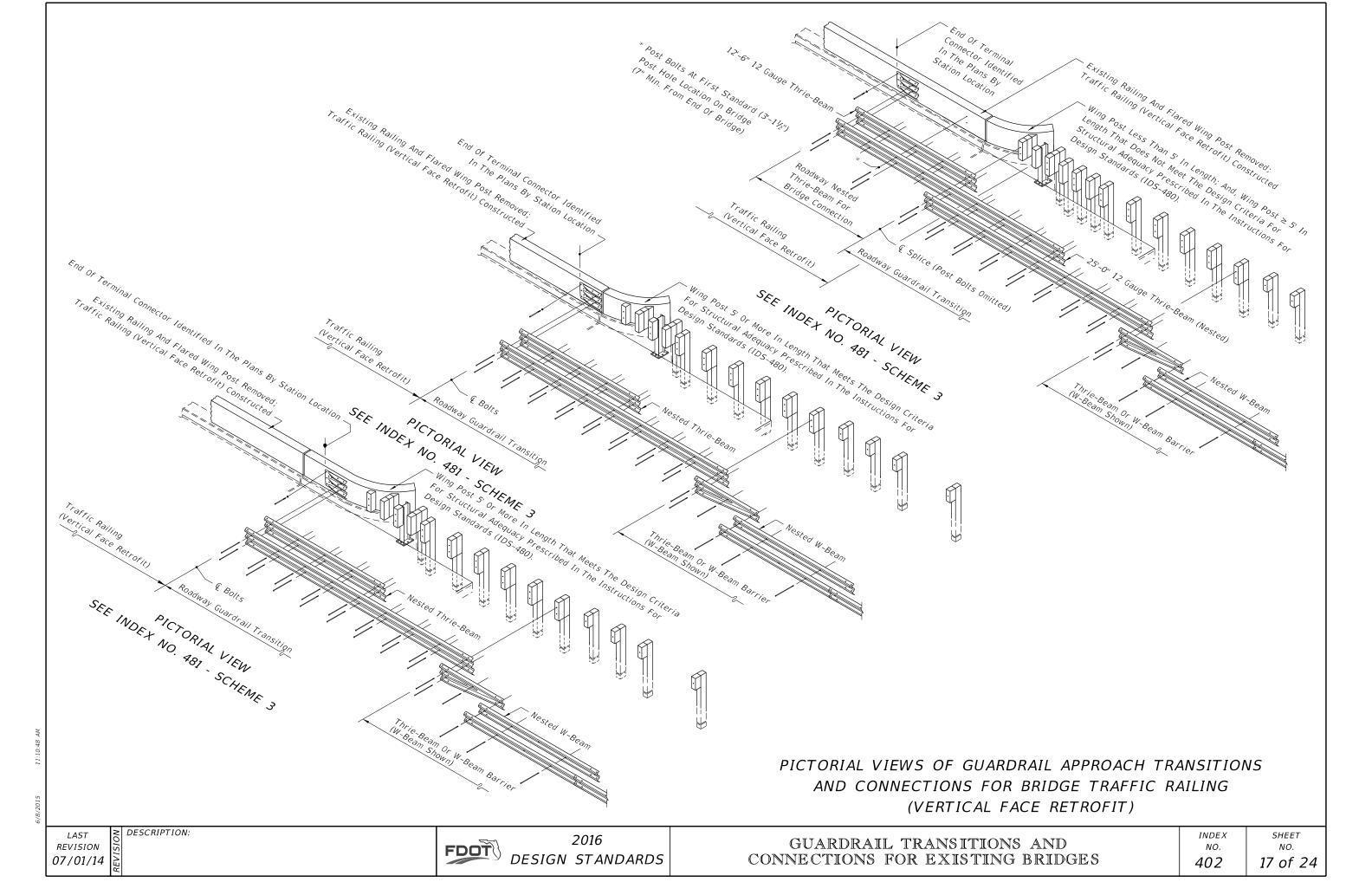
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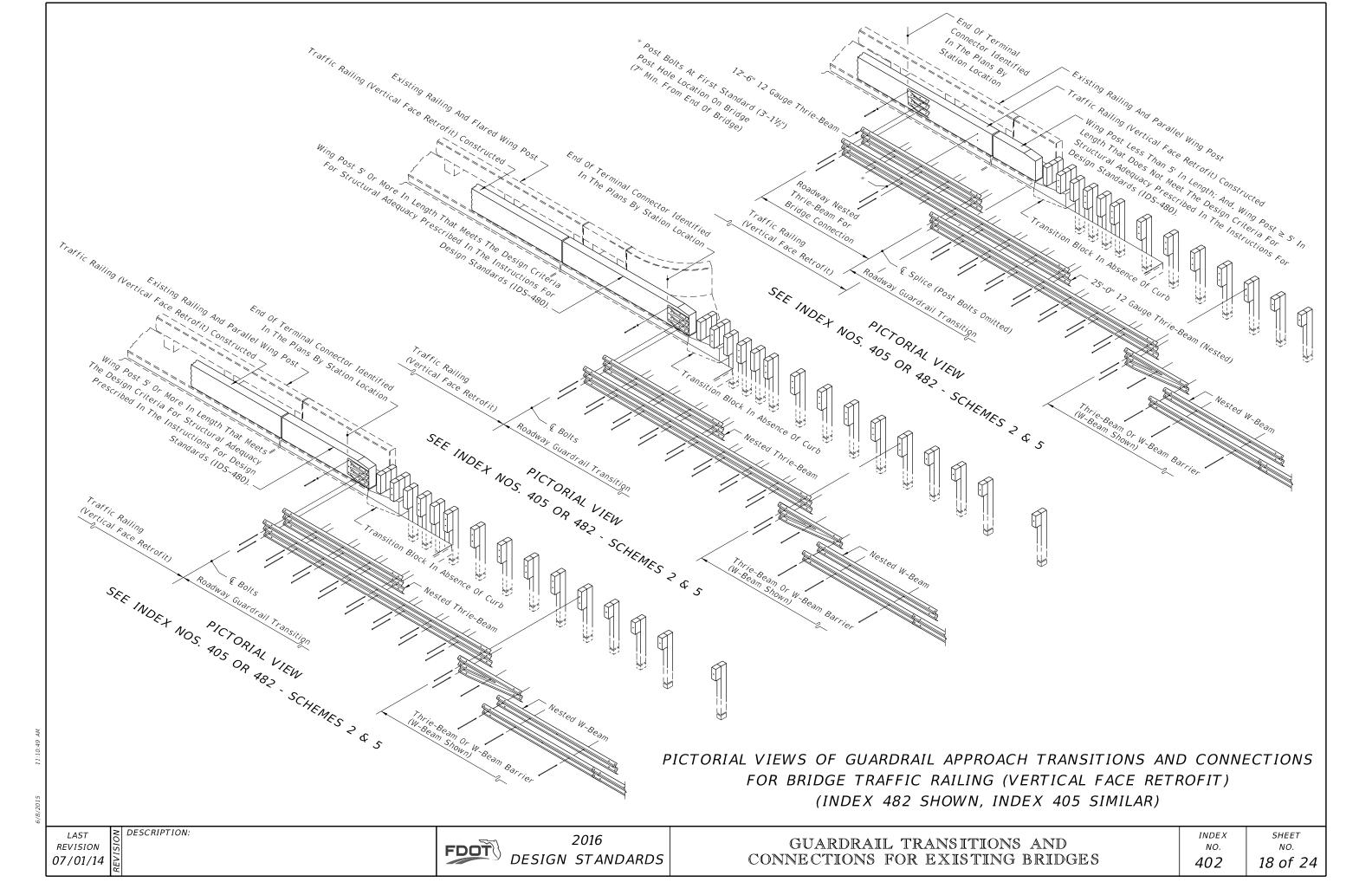


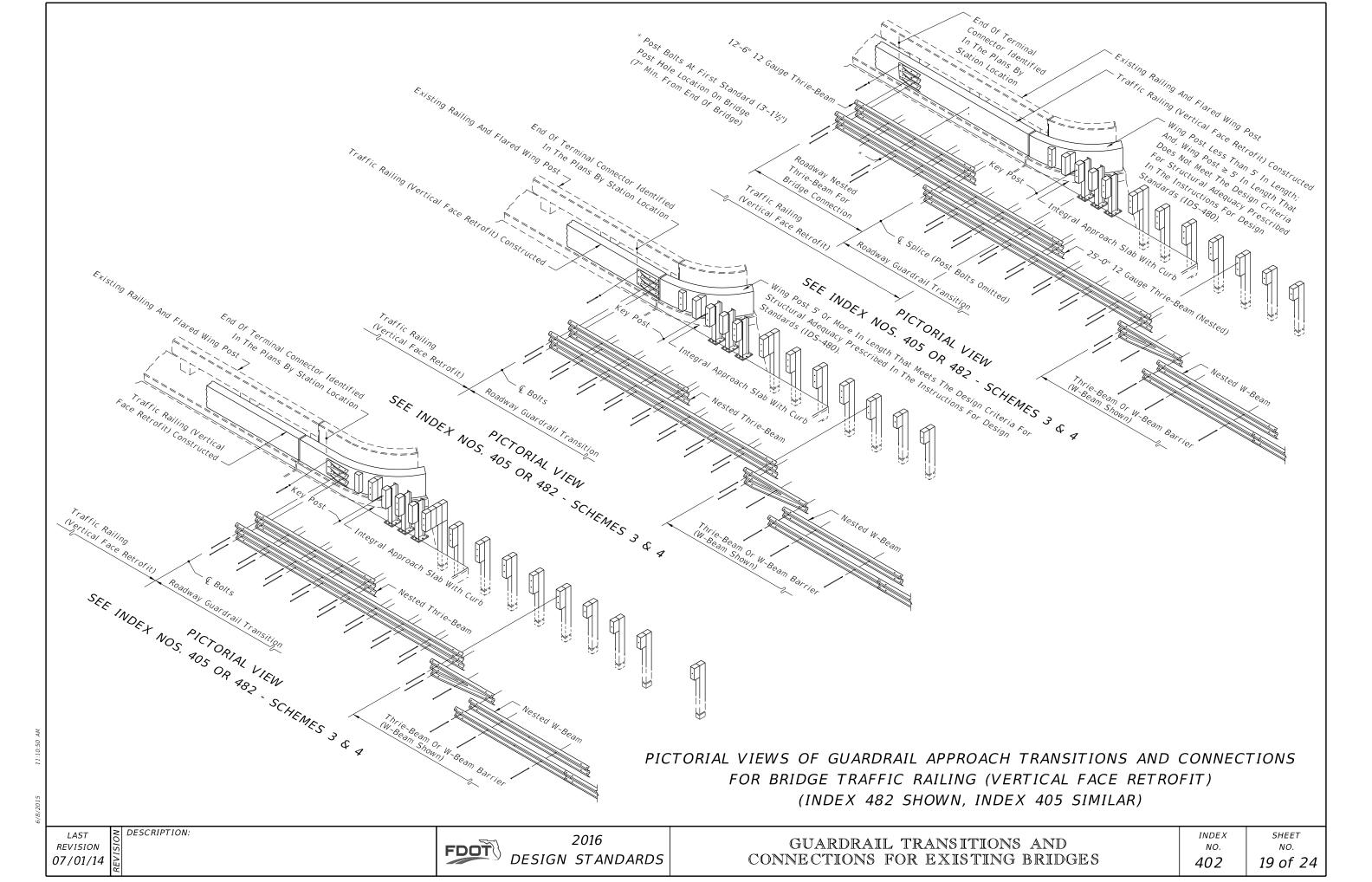


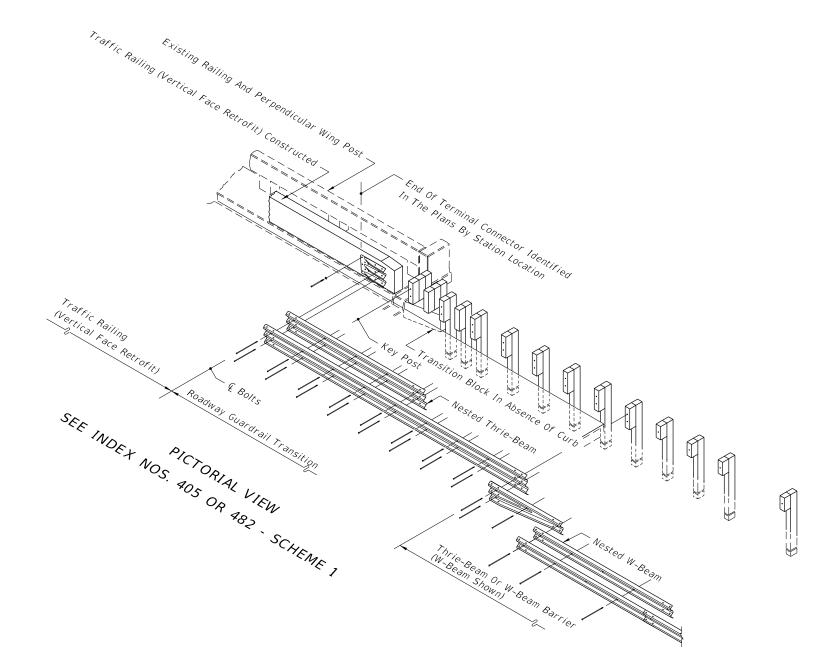






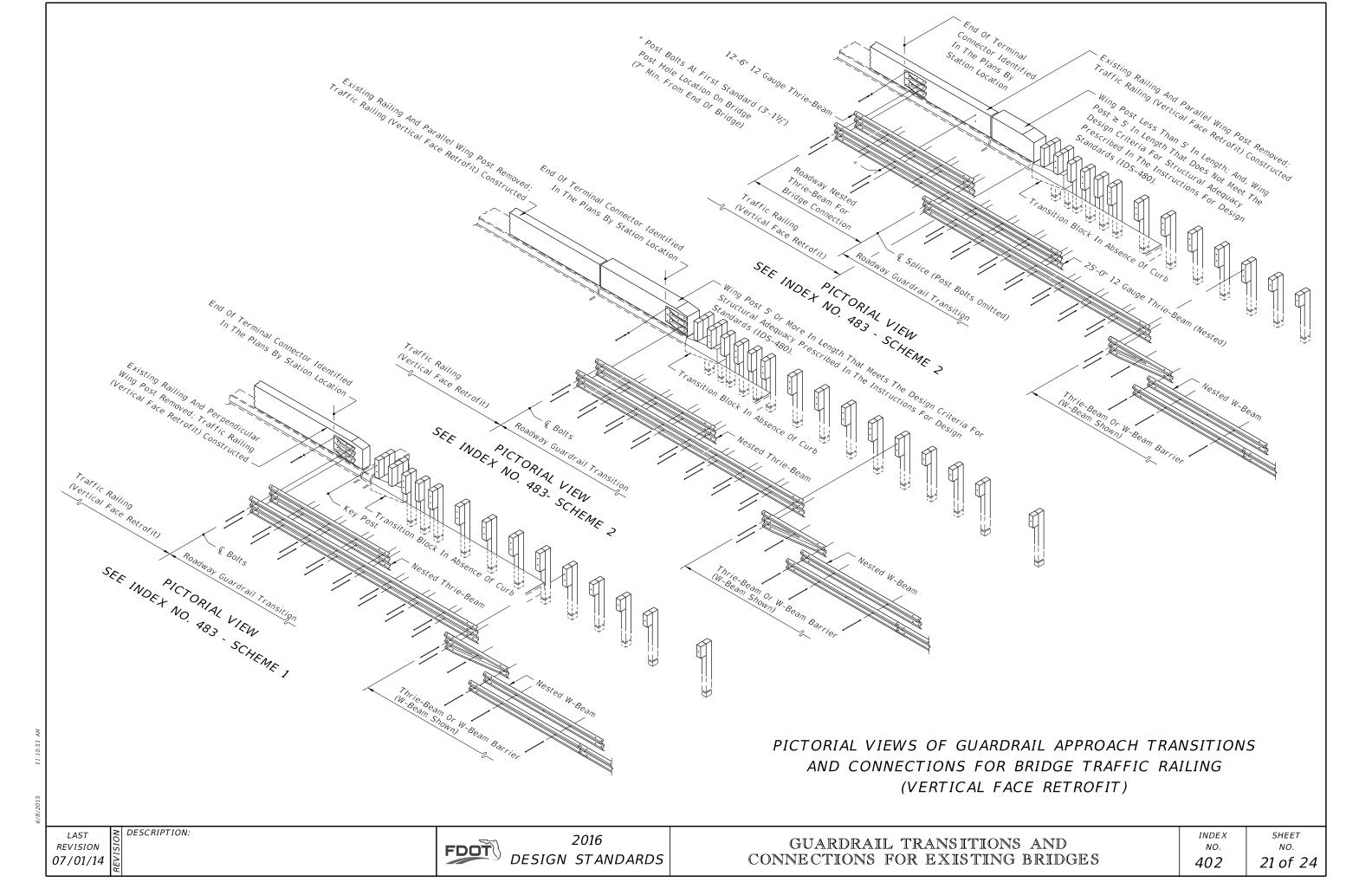


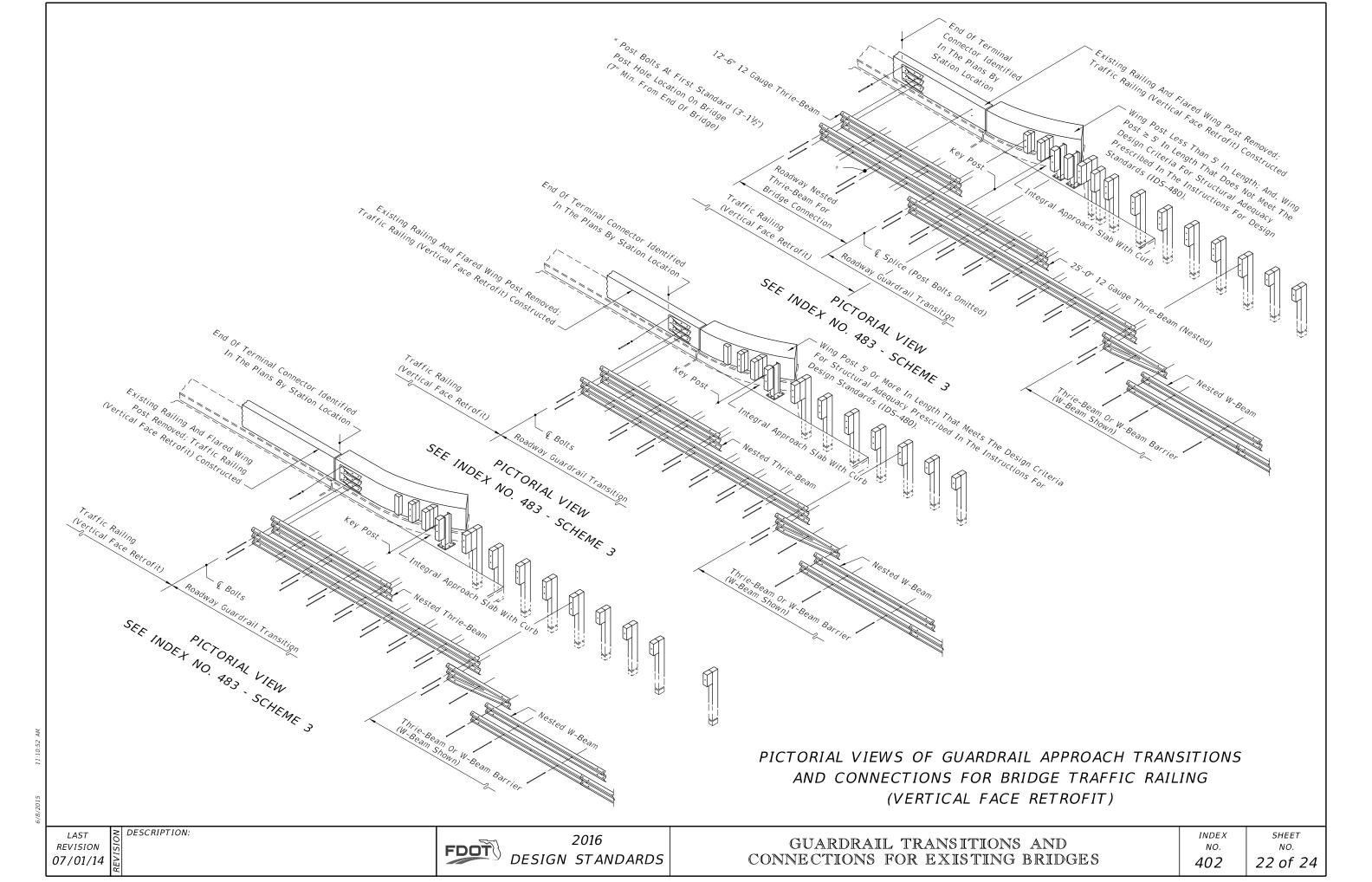


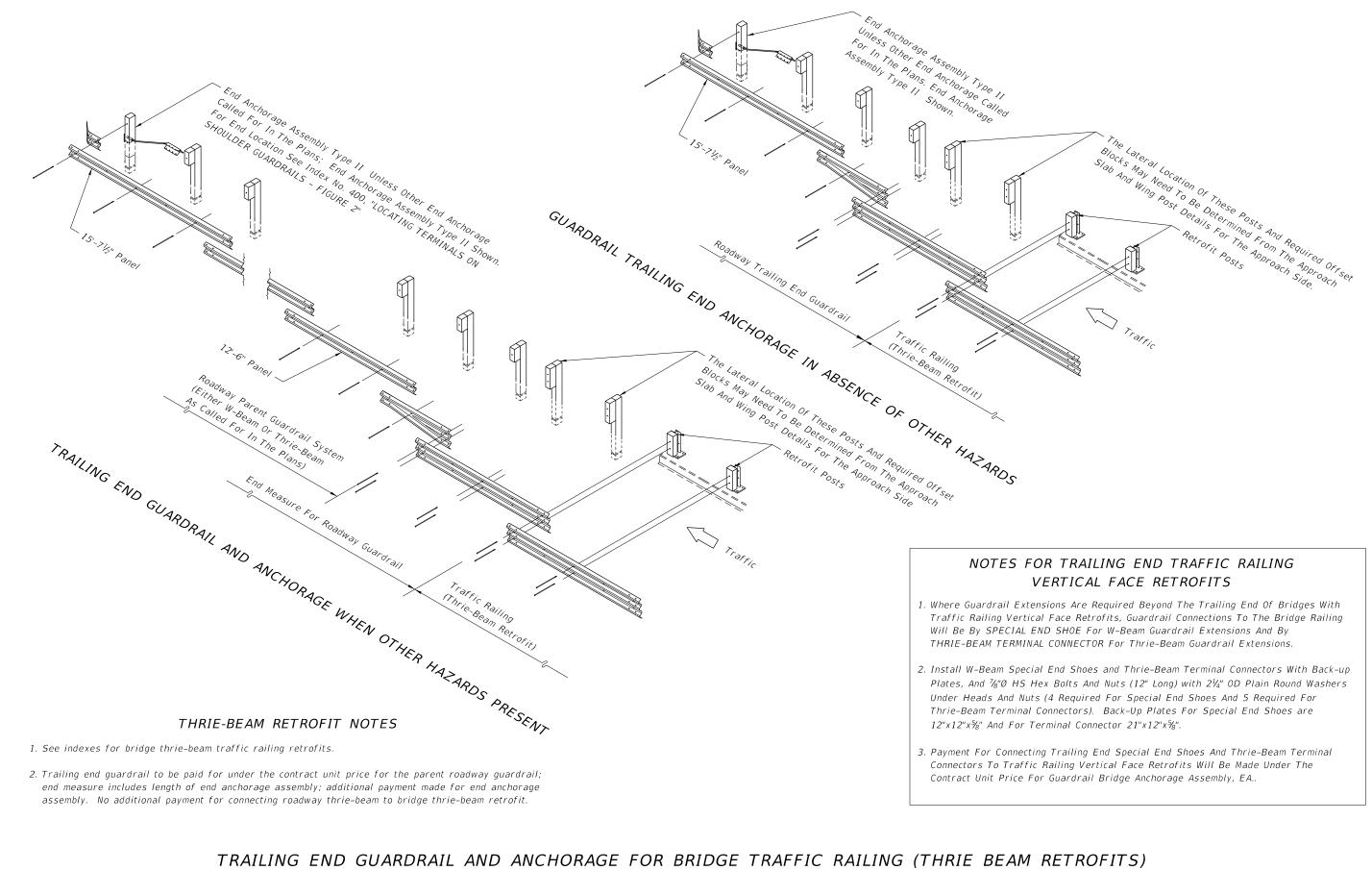


PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 482 SHOWN, INDEX 405 SIMILAR)

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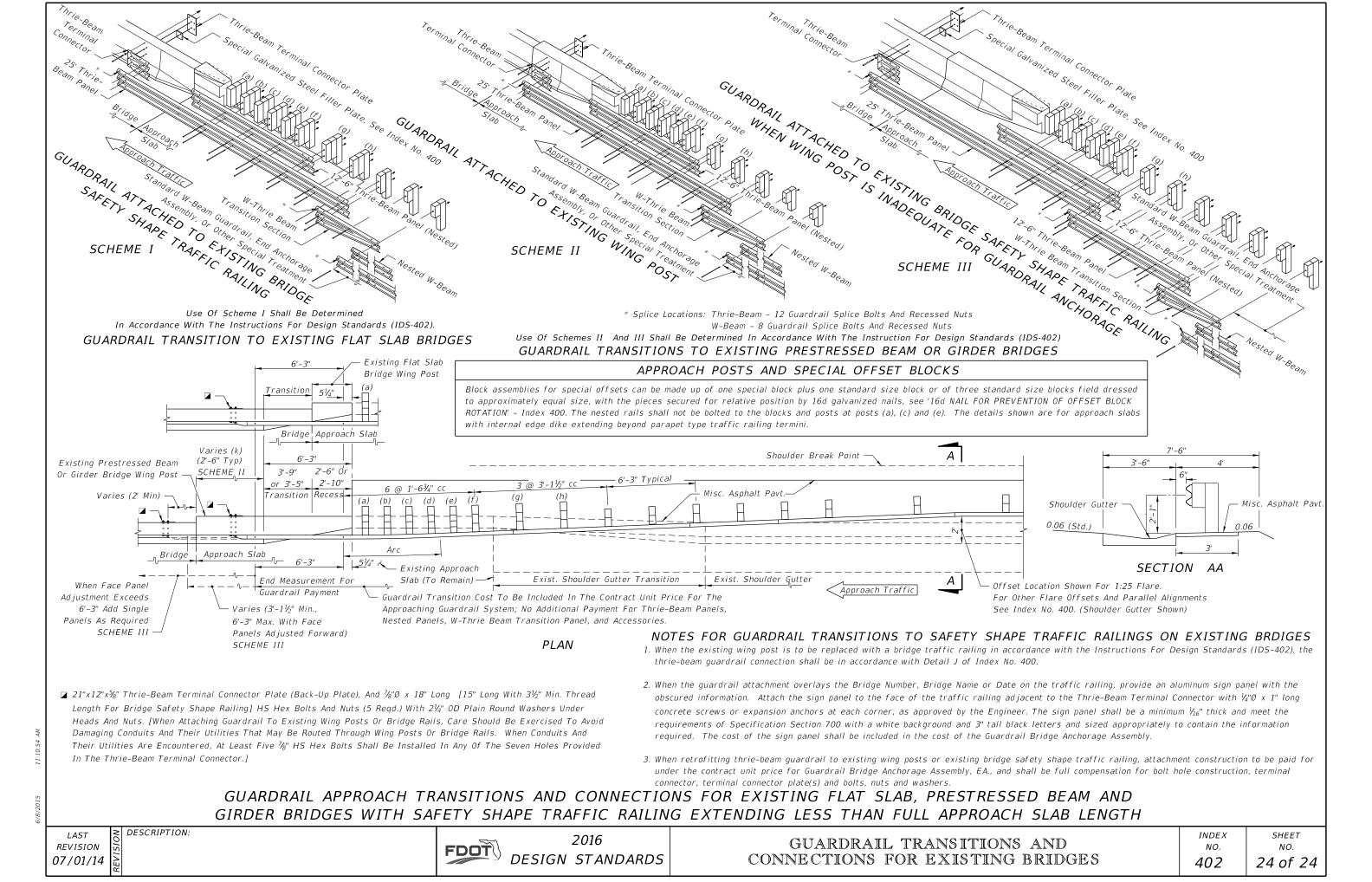


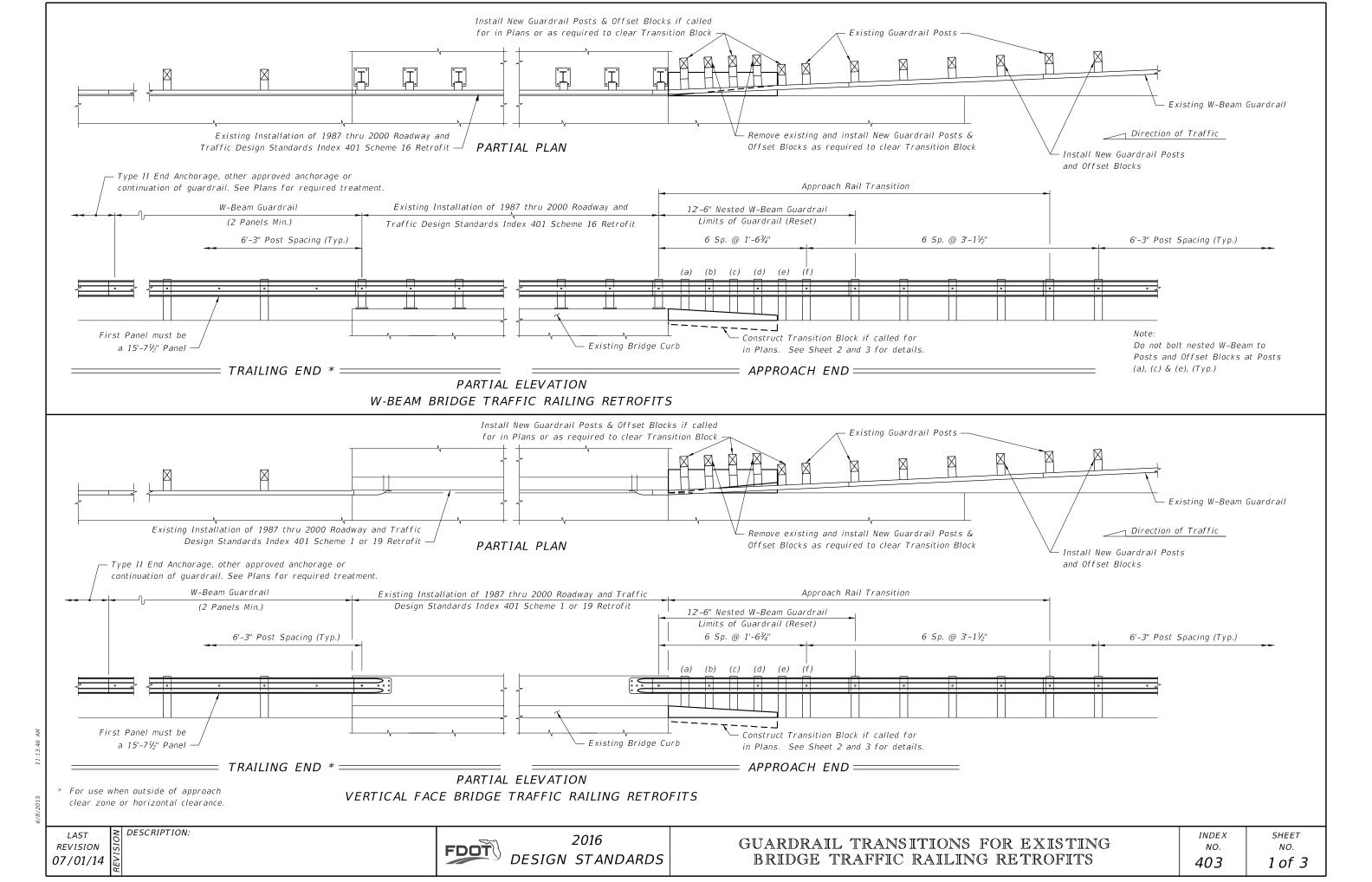
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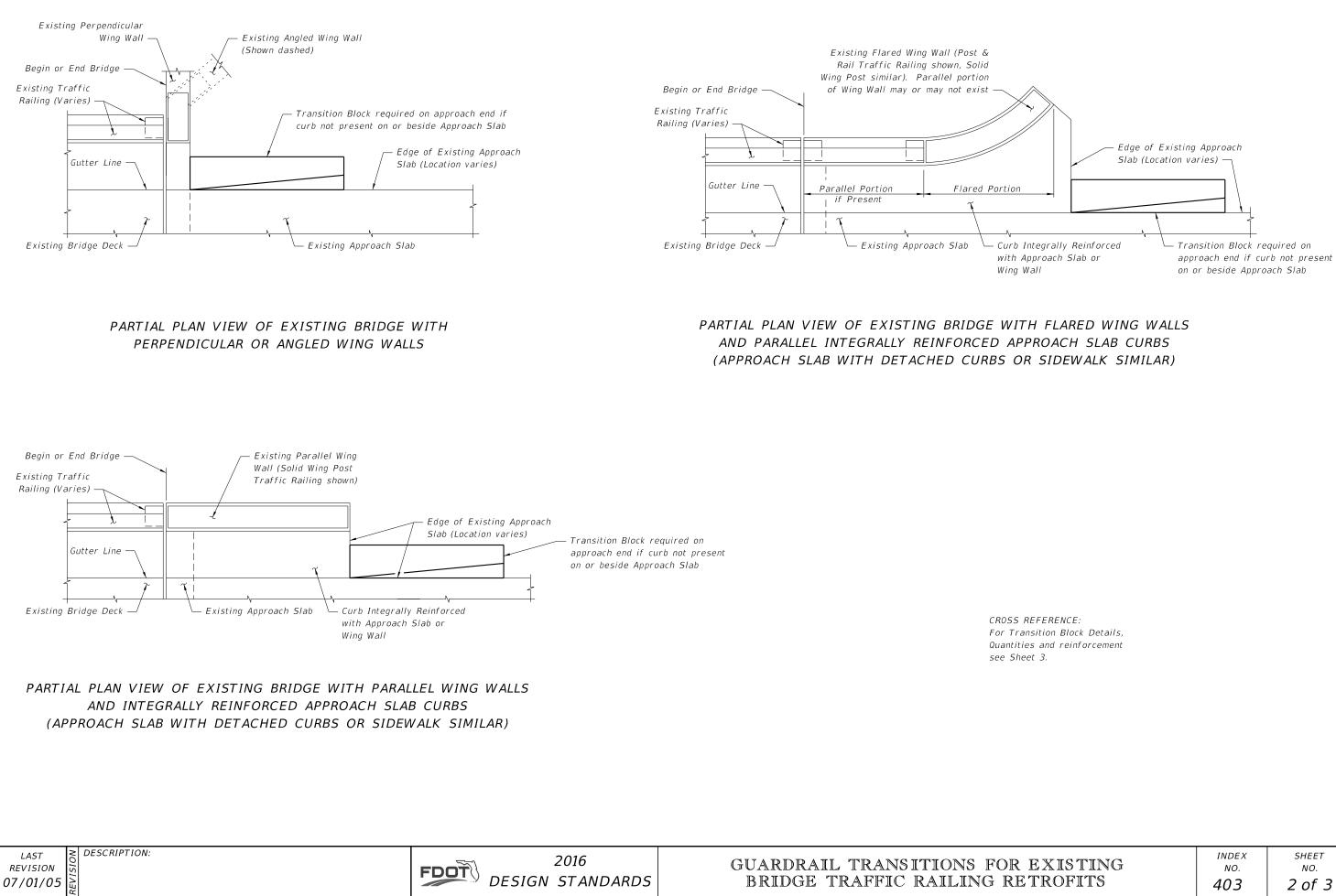


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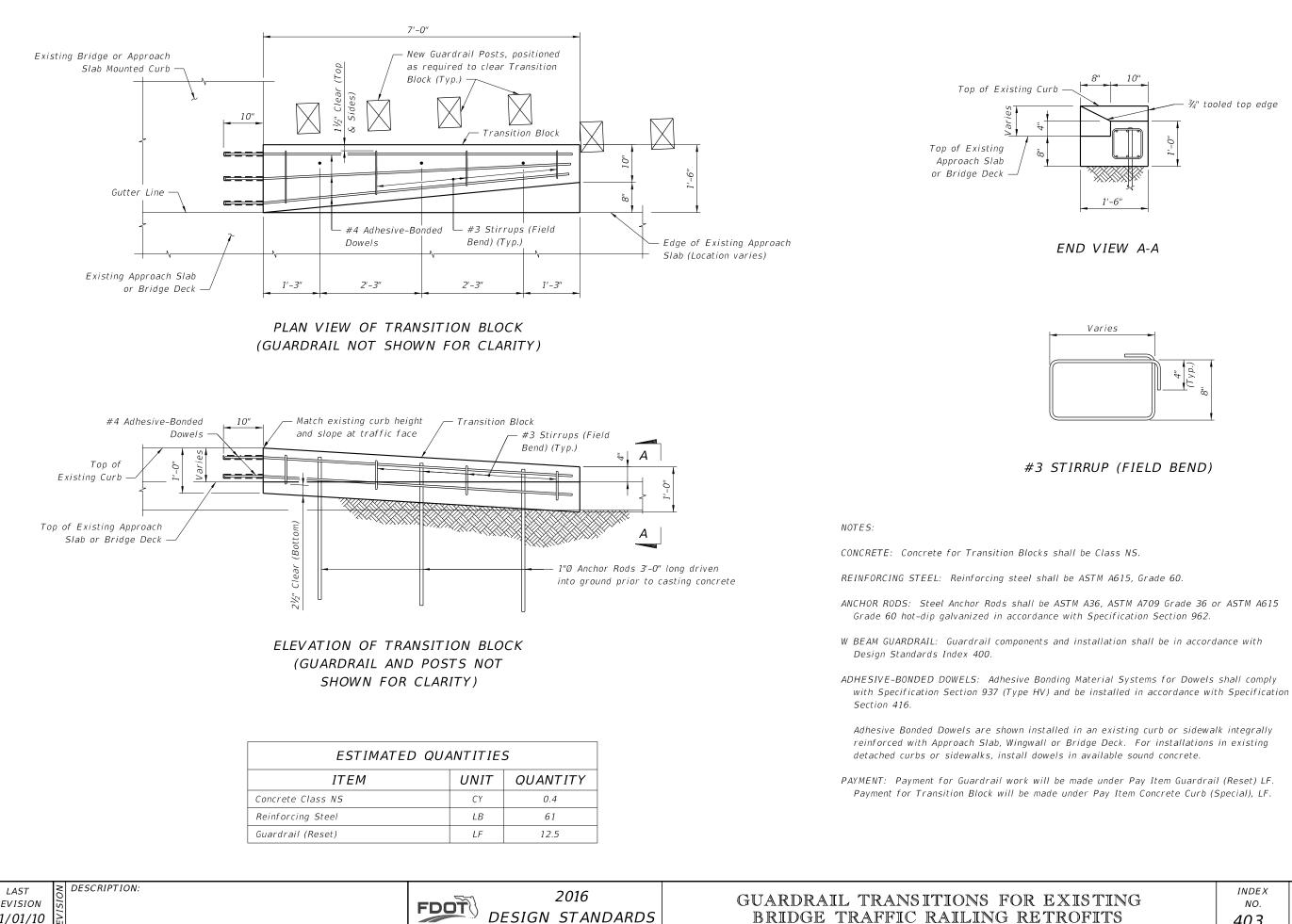
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BRIDGE TRAFFIC RAILING RETH

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### GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

BRIDGES ON CURVED ALIGNMENTS: The details presented in this Standard are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing along the entire length of the bridge 2" from the face on the traffic side at the spacing shown in the table below. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

GUARDRAIL: See Index 400 for guardrail component details, geometric layouts and associated notes not fully detailed herein.

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent quardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise individual decals of letters and numbers.

PAYMENT: Guardrail Bridge Anchorage Assembly (each) includes all barrier delineators for the entire bridge length, transition blocks, and necessary hardware to complete the Guardrail transitions shown.

Hole Diameter to meet

Distance -

Edge of Travel Lane

4' to 8' > than 8'

to Face of Railing < 4'

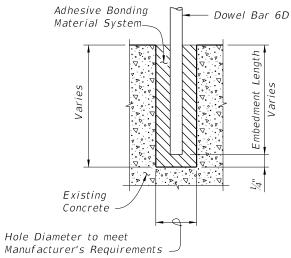
BARRIER DELINEATOR SPACING

Spacing (Ft.)

40' 80'

None Required

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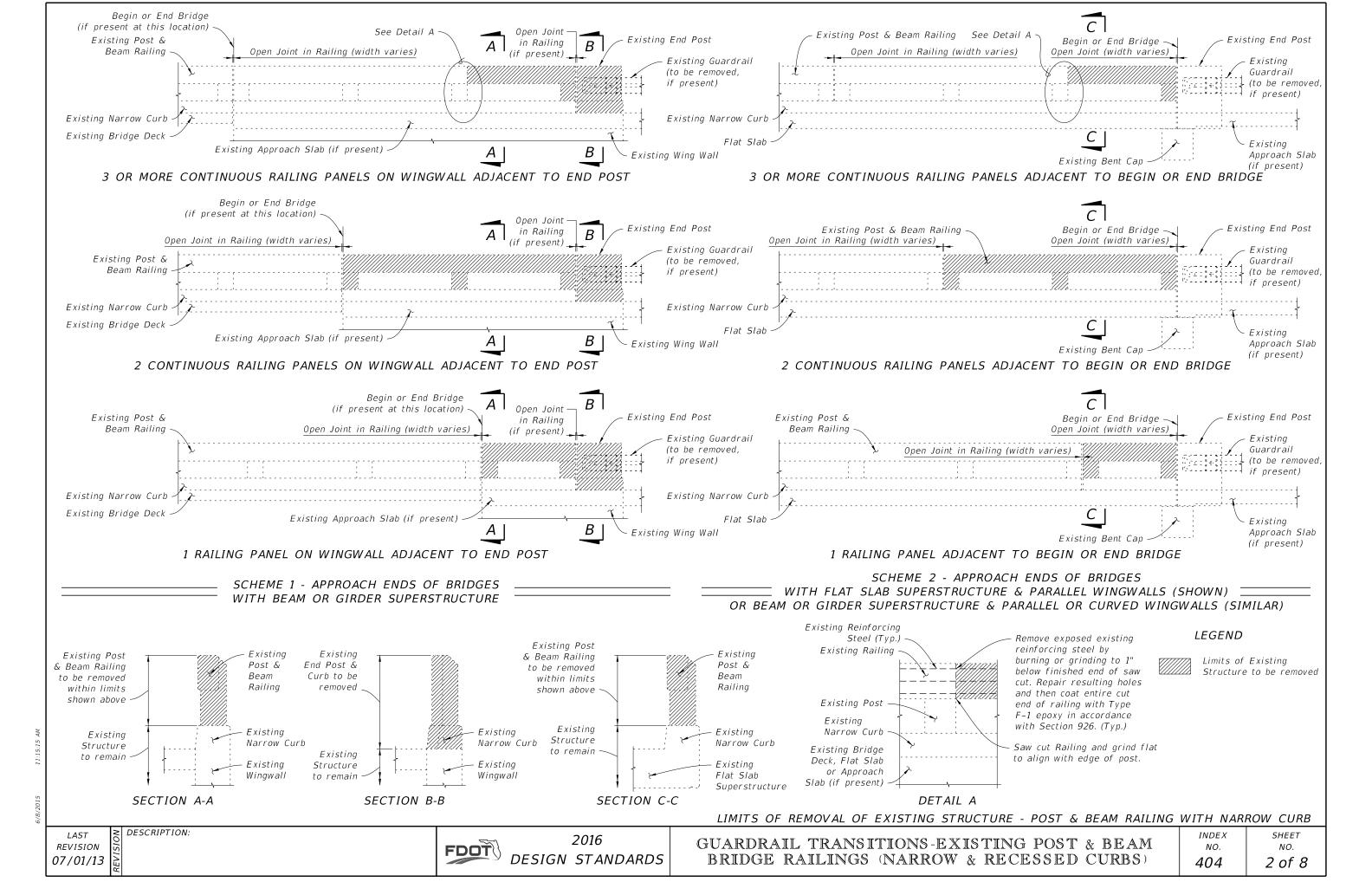


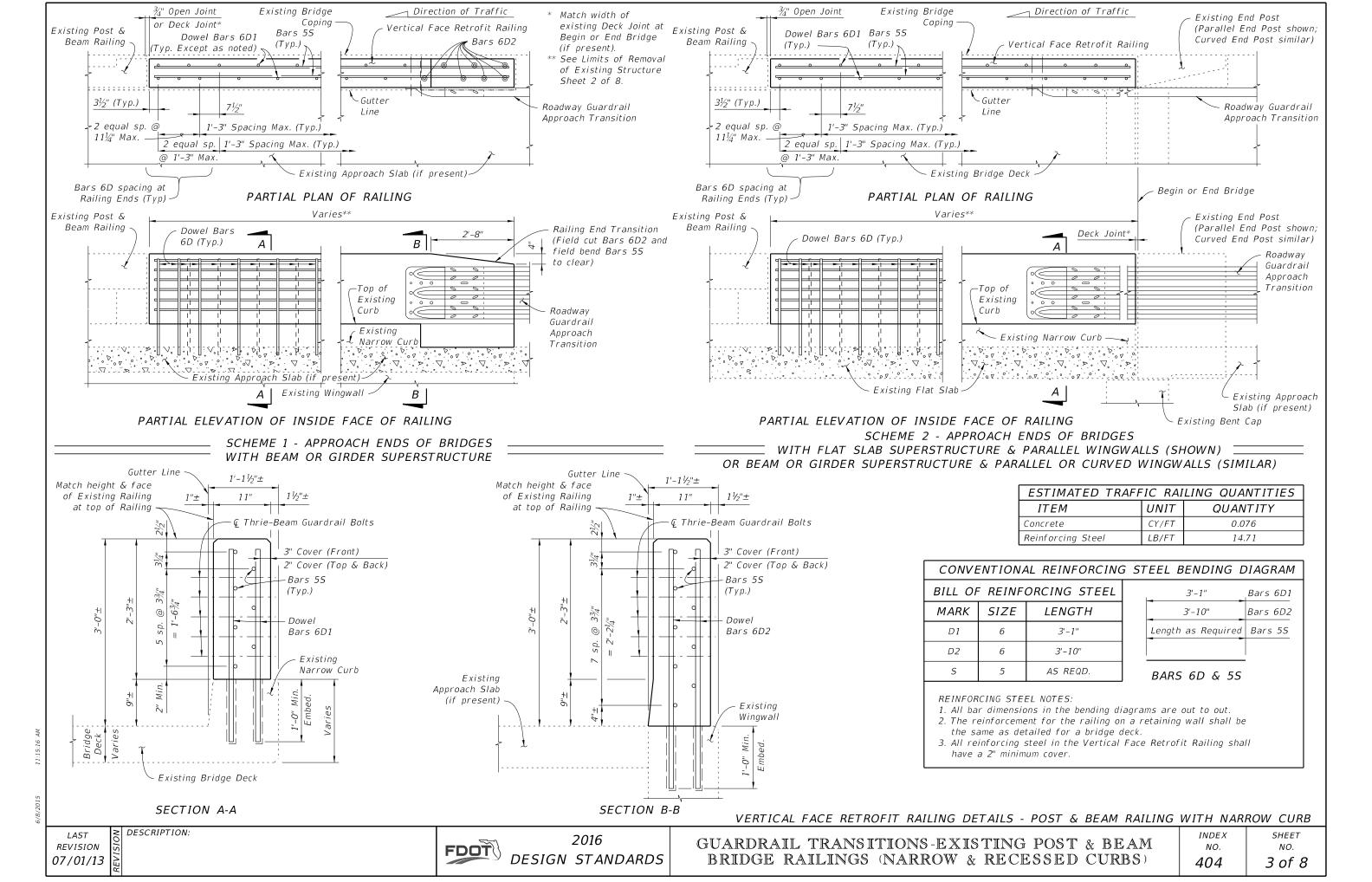
DOWEL DETAIL

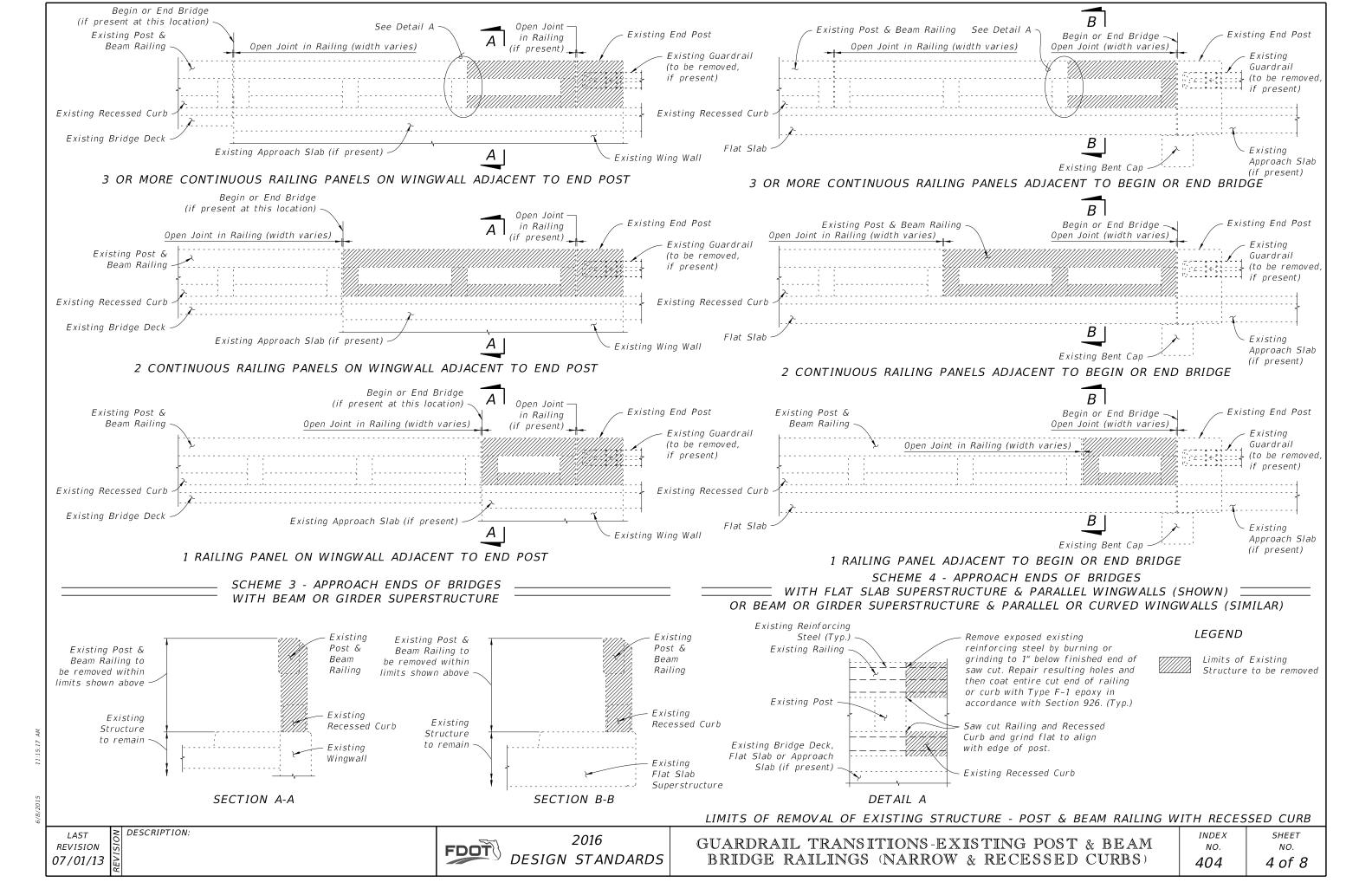
Note:

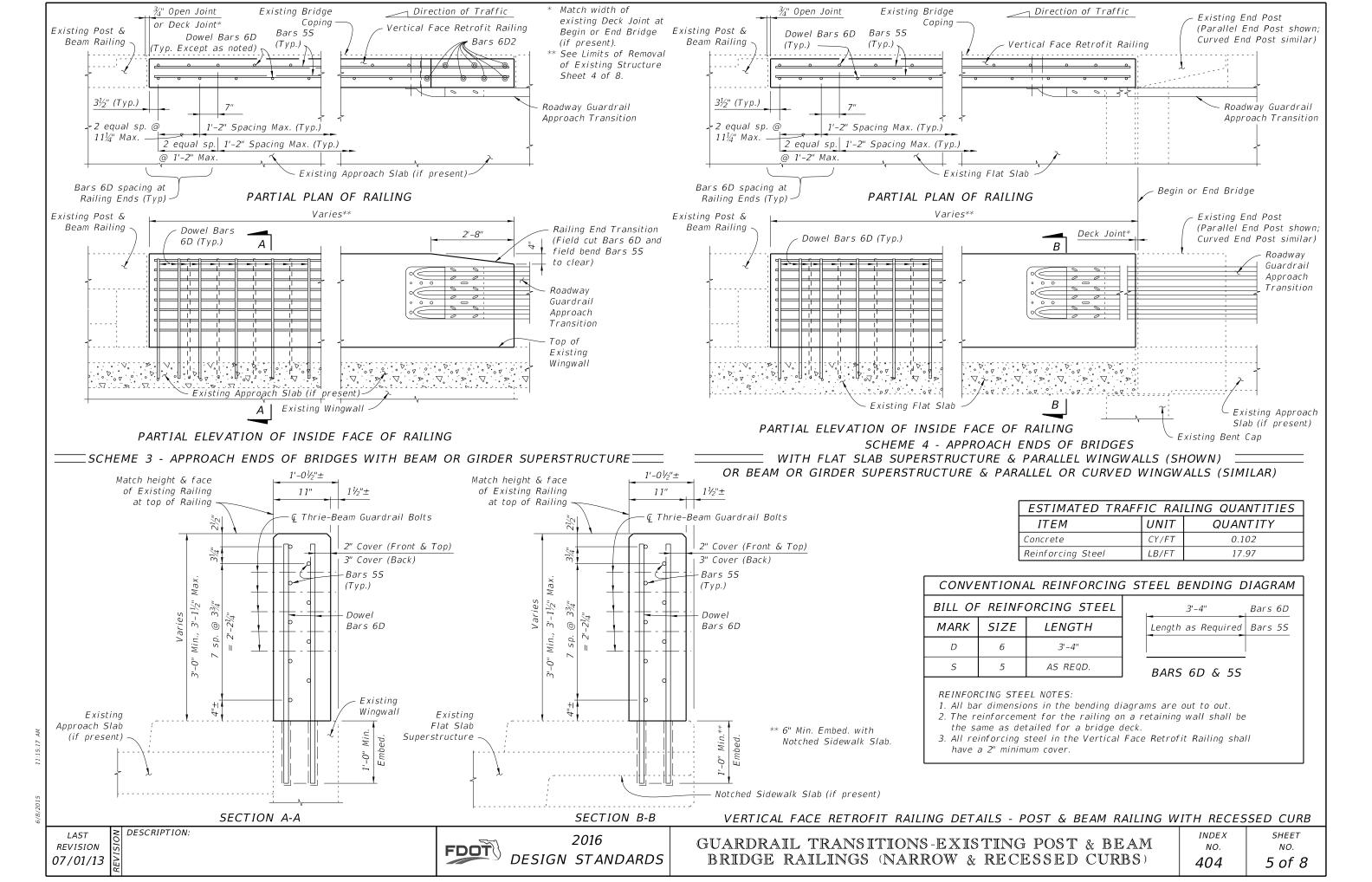
Shift dowel holes to clear if the existing reinforcement is encountered.

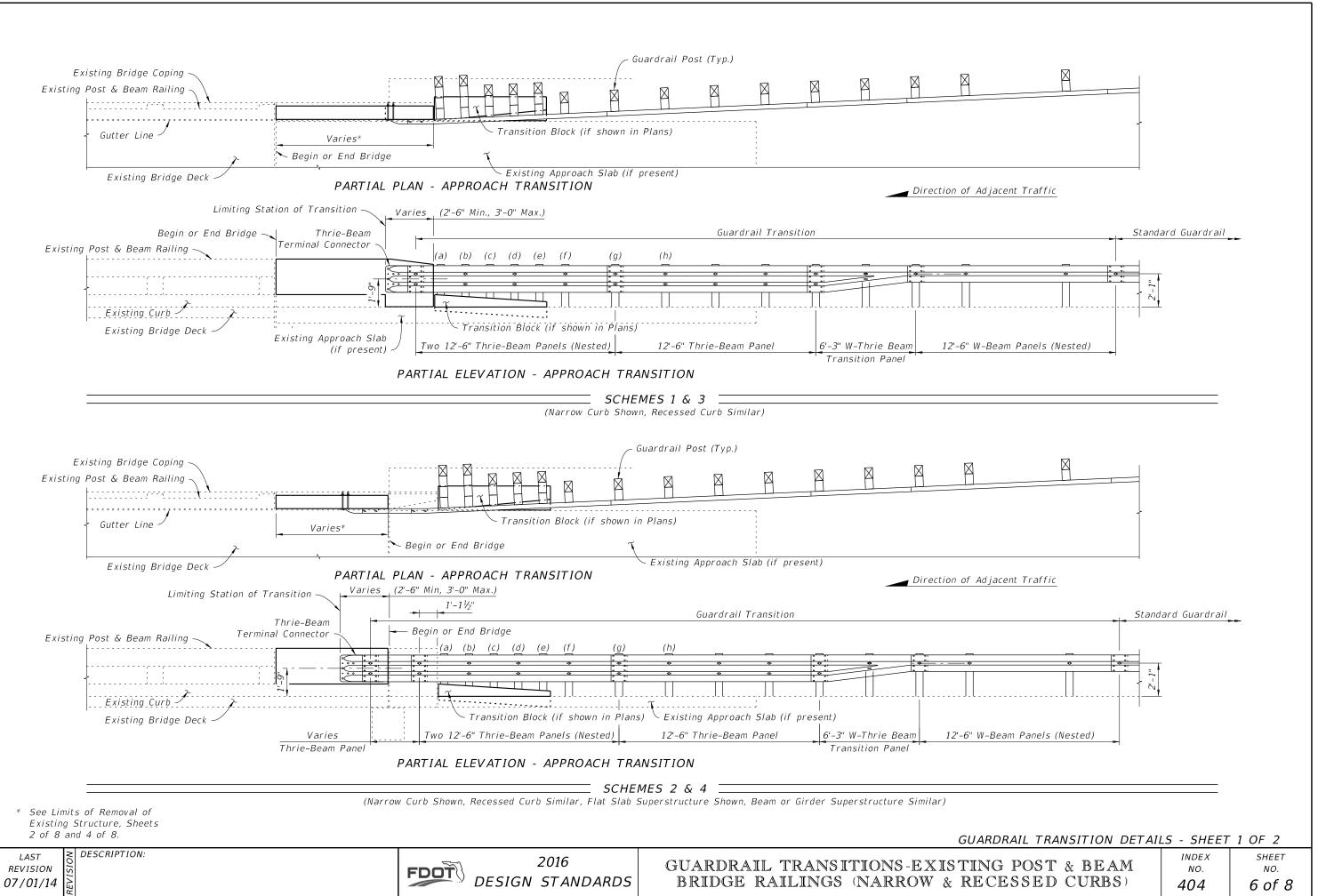
POST & BEAM SSED CURBS)	index no. <b>404</b>	<sup>sheet</sup> NO. <b>1 of 8</b>

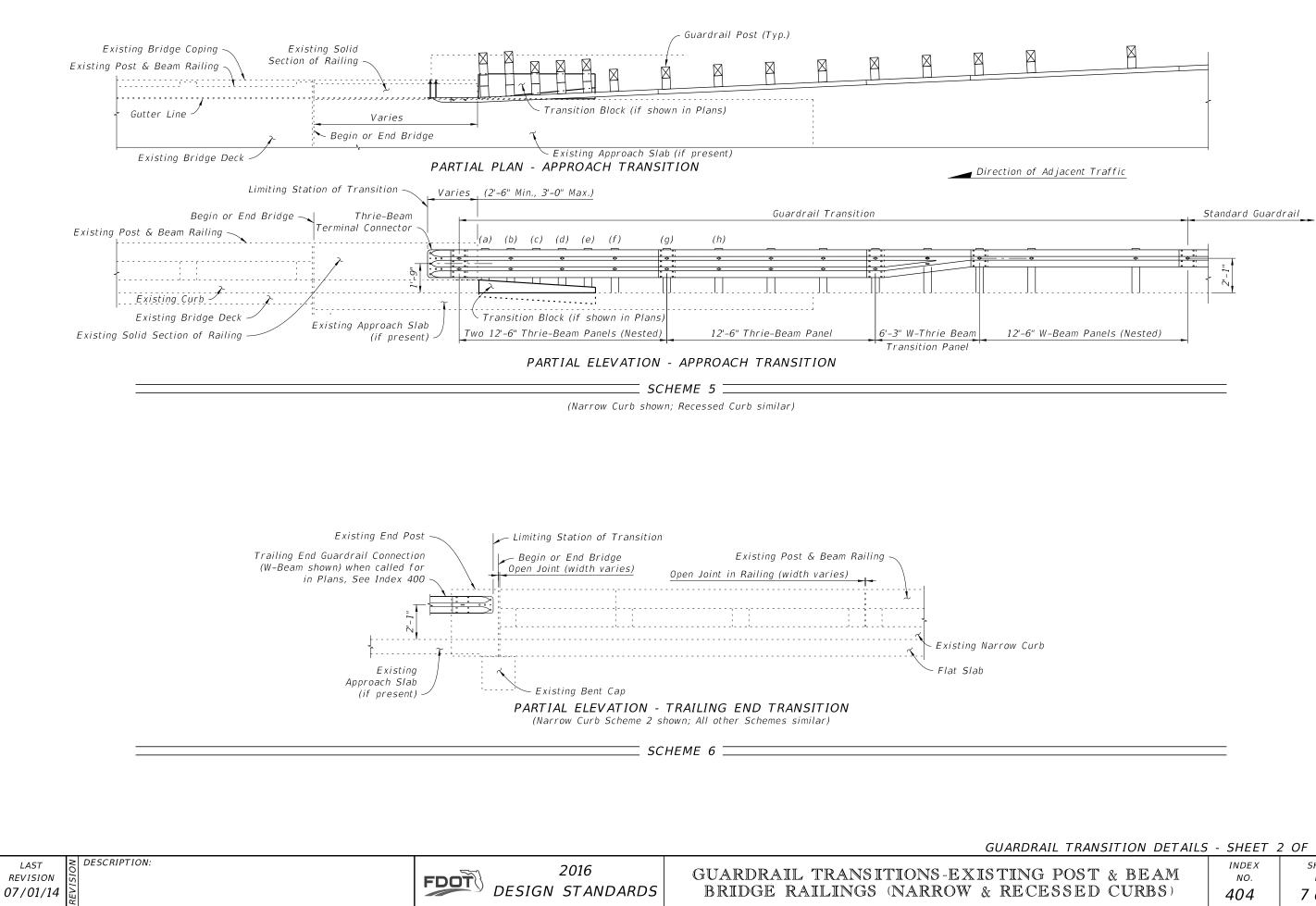




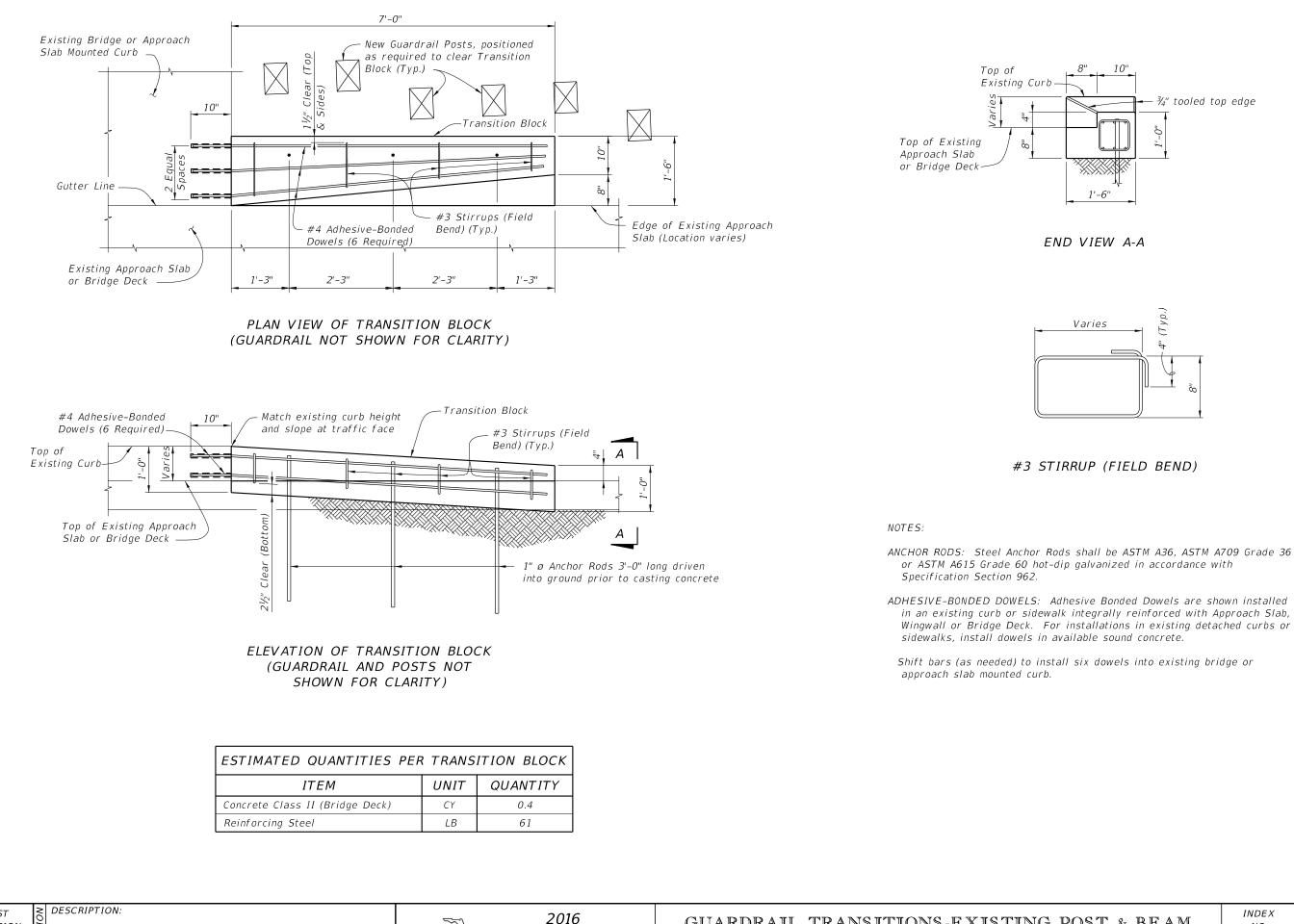








TRANSITION DETAILS	- SHEET 2	2 OF 2
POST & BEAM SSED CURBS)	index no. <b>404</b>	<sup>sнеет</sup> NO. <b>7 of 8</b>



GUARDRAIL TRANSITIONS-EXISTING F BRIDGE RAILINGS (NARROW & RECES

POST & BEAM SED CURBS)	index no. <b>404</b>	<sup>SHEET</sup> NO. <b>8 of 8</b>

Match Deck Joint width

### GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615. Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

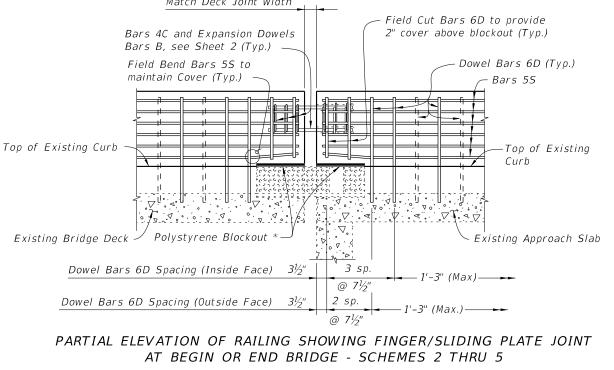
EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

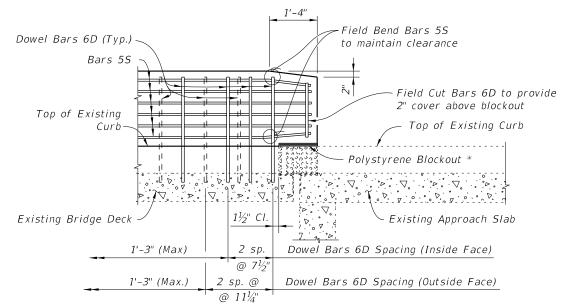
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install barrier delineators on top of the Traffic Railing along the entire length of bridge 2" from the face on the traffic side at the spacing shown in the table below. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Concrete Traffic Railing- Bridge Retrofit- Post & Beam Railing (each) includes all materials and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railings. Guardrail Bridge Anchorage Assembly (each) includes all barrier delineators for the entire bridge length, transition blocks, and necessary hardware to complete the Guardrail transitions shown.



\* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



JOINT AT BEGIN OR END BRIDGE - SCHEME 1 (Guardrail Transition not shown for clarity)

BARRIER DELINEATOR SPACING Distance – Spacing (Ft.) Edge of Travel Lane to Face of Railing < 4' 40' 4' to 8' 80' > than 8' None Required

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
	UNIT	9" Curb	Increment
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.)

DESCRIPTION: LAST REVISION 07/01/13

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2016 DESIGN STANDARDS

GUARDRAIL TRANSITIONS - EXI POST & BEAM BRIDGE RAILINGS (WI

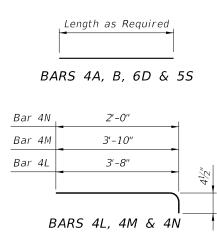


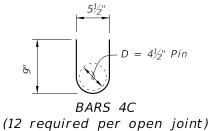
# PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE

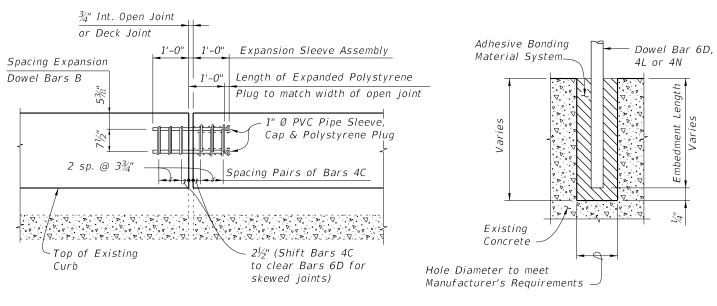
ISTING	INDEX NO.	SHEET NO.
IDE CURBS)	405	1 of 6

	υ	NVENTIONAL	REINFORCING .	STEEL BENDING	g diagram
В	BILL OF	REINFORCIN	G STEEL		
MARK	SIZE	LENGTH	NOTE NOS.		ength as Requi
A	4	AS REQD.	3	-	
В	1" Ø	2'-0"	2 & 5	BAR	S 4A, B, 6D
С	4	2'-0"	1, 2 & 3		
D	6	AS REQD.	2&3	Bar 4N	2'-0"
L	4	4'-1"	1 & 3	Bar 4M	3'-10''
М	4	4'-3''	1 & 3	Bar 4L	3'-8"
N	4	2'-5"	1&3	1 –	
5	5	AS REQD.	2, 3 & 4	ВД	RS 4L, 4M 8

- REINFORCING STEEL NOTES:
- 1. All bar dimensions in the bending diagrams are out to out. 2. The reinforcement for the railing on a retaining wall shall
- be the same as detailed for a bridge deck. 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.

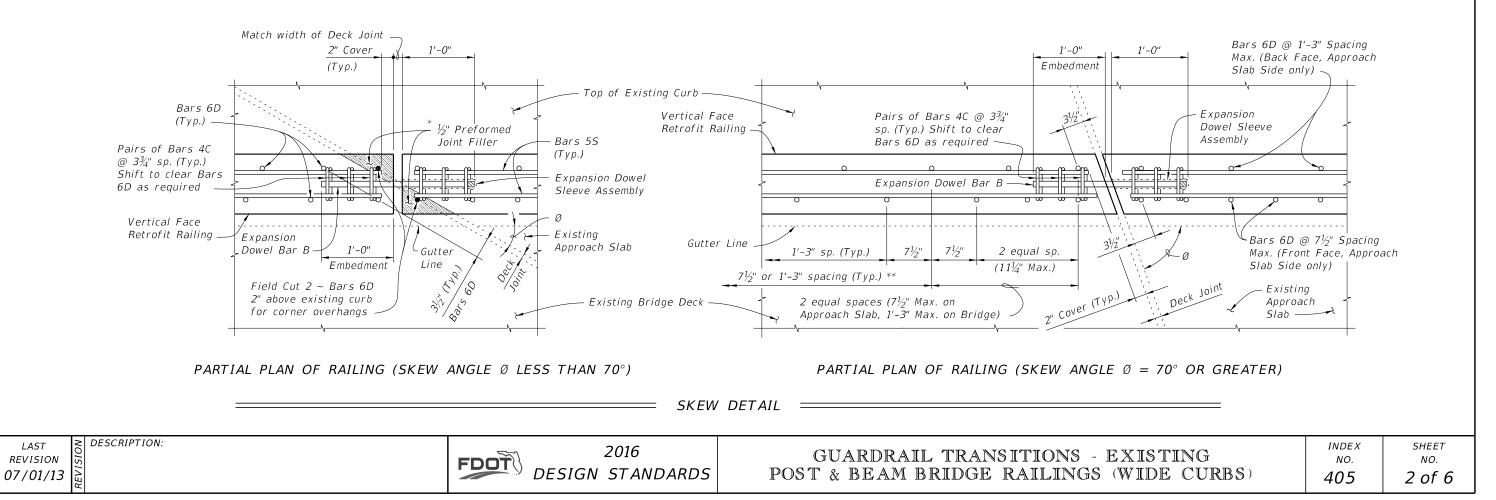






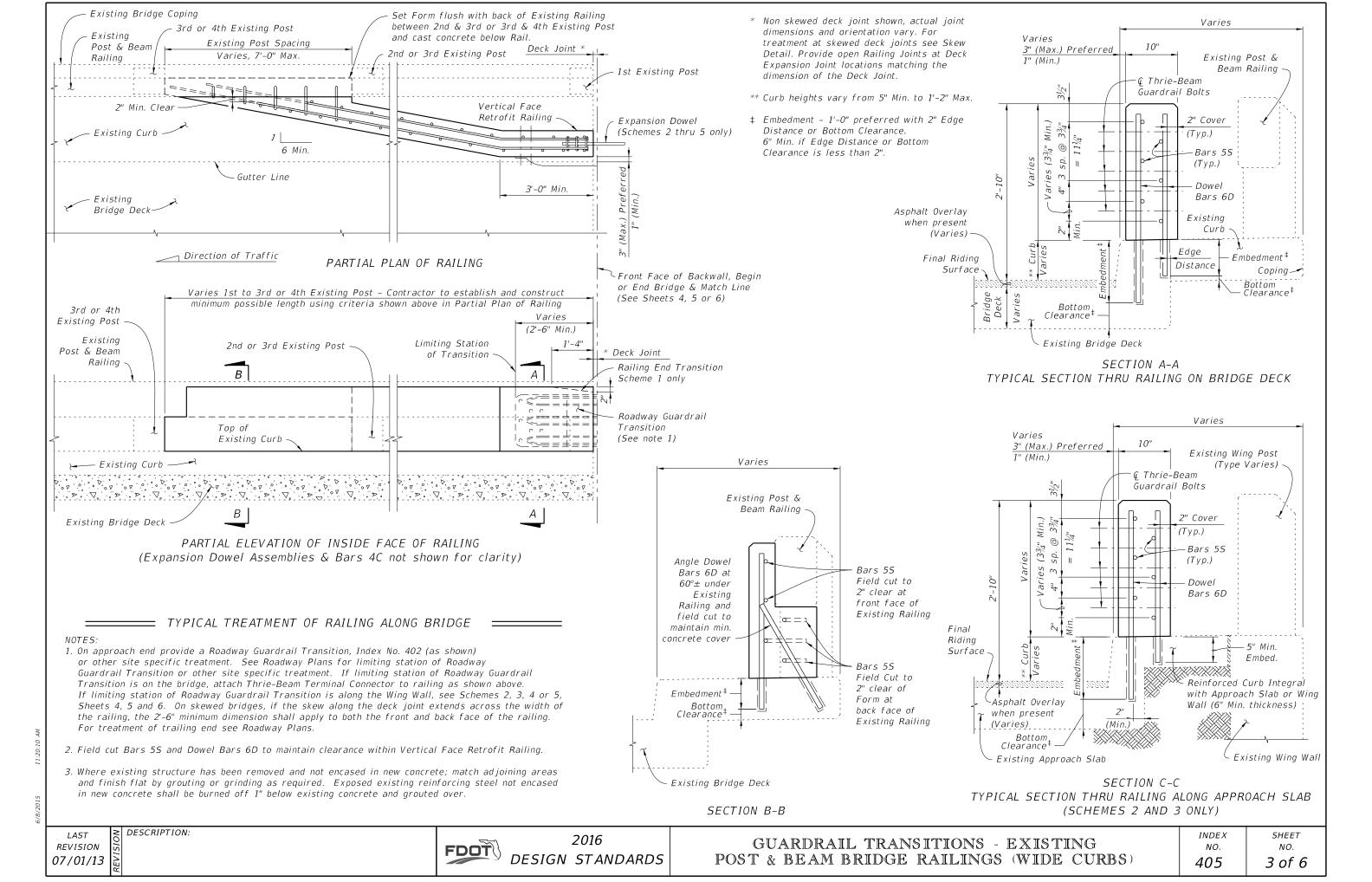
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)

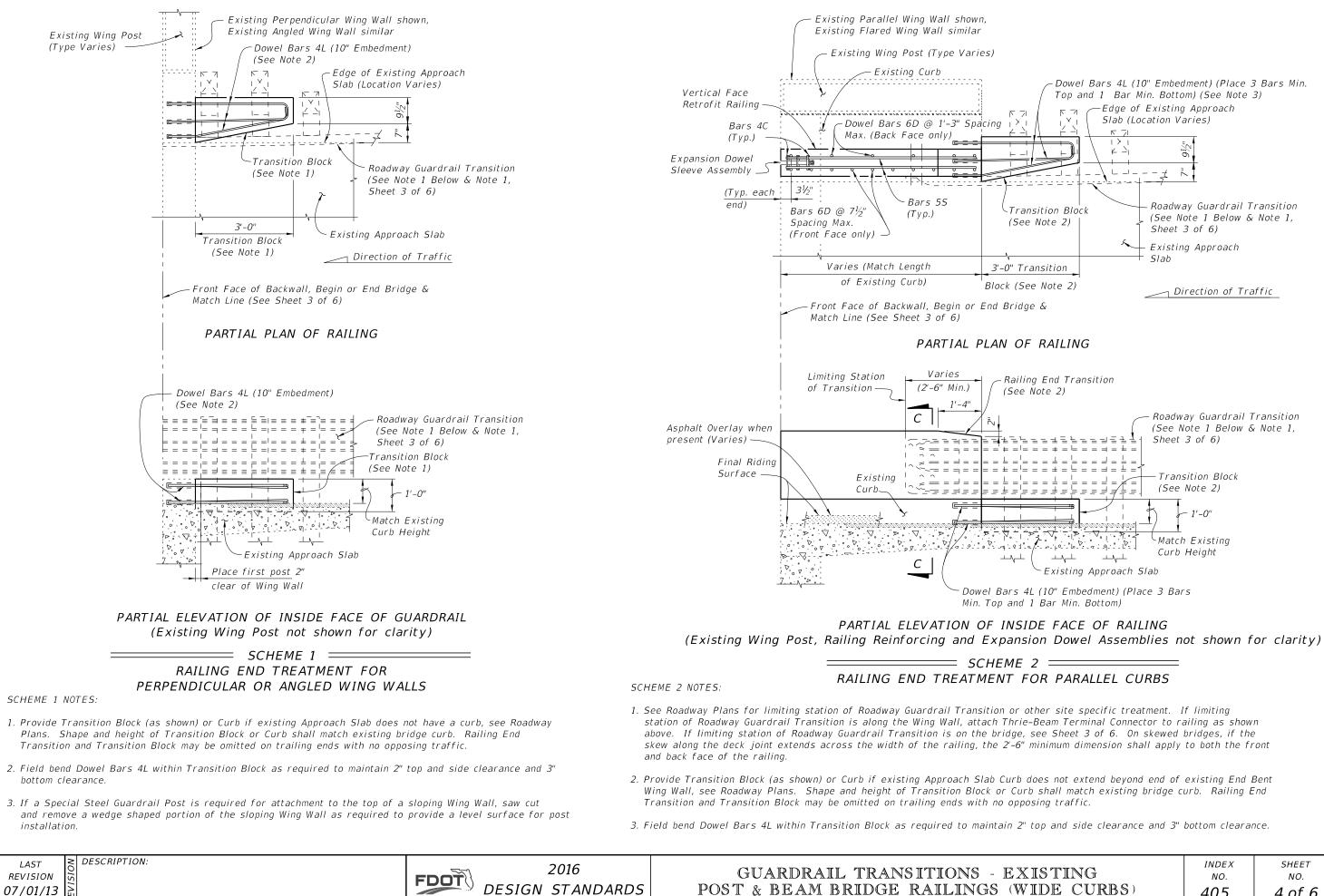
\* ½" Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.



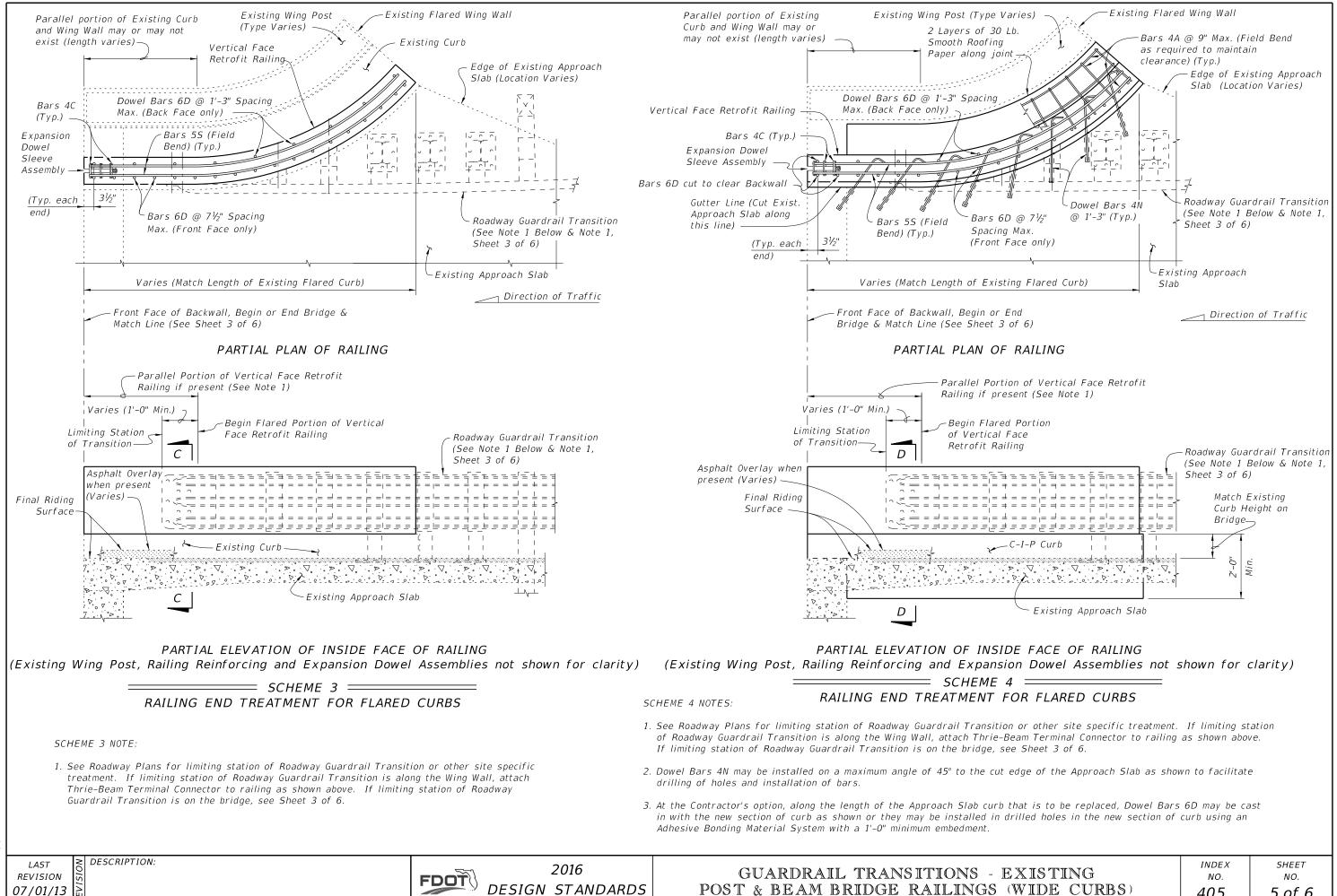
### DOWEL DETAIL

Dowel Installation Note: Shift dowel holes to clear if the existing reinforcement is encountered.

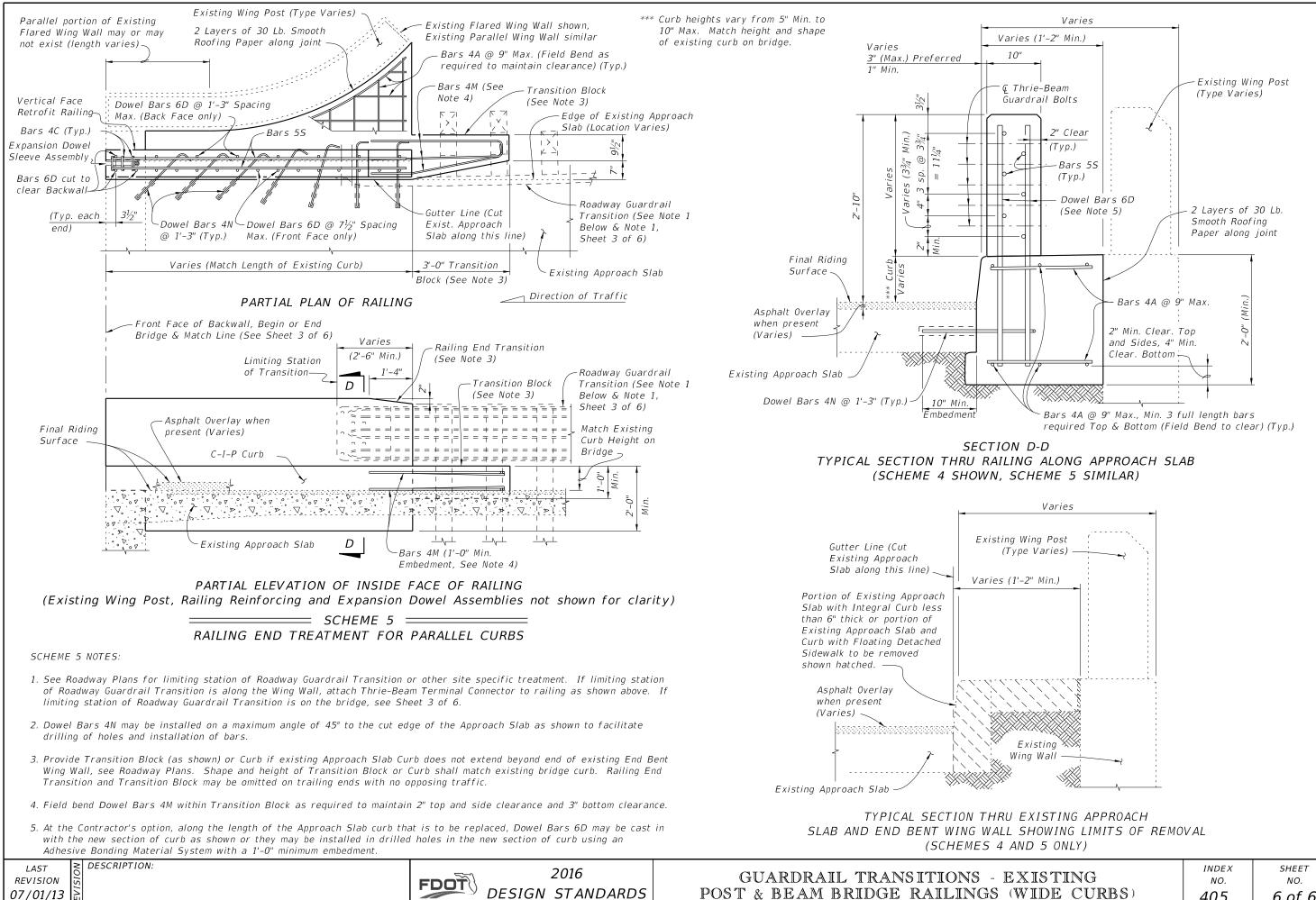




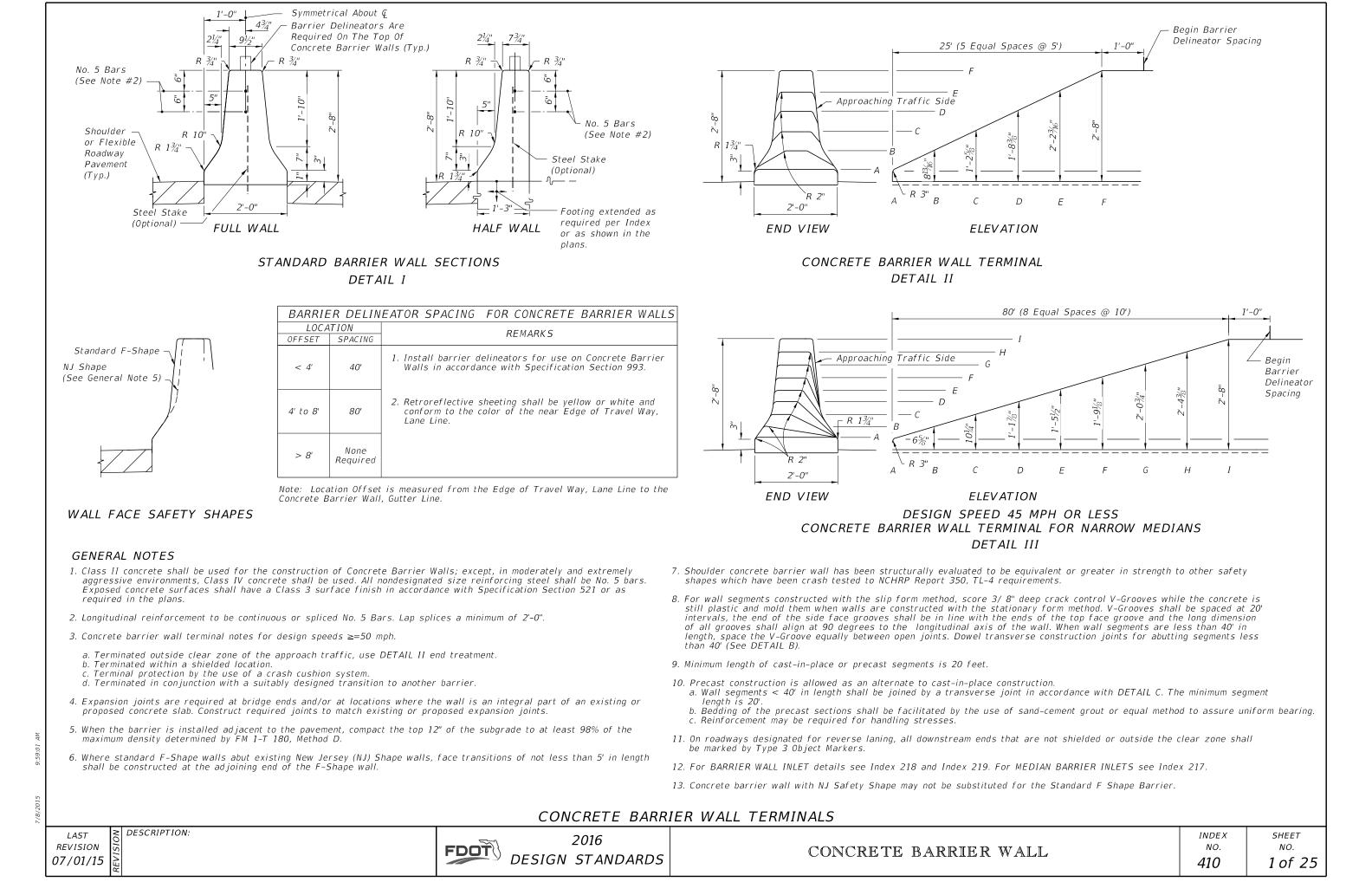
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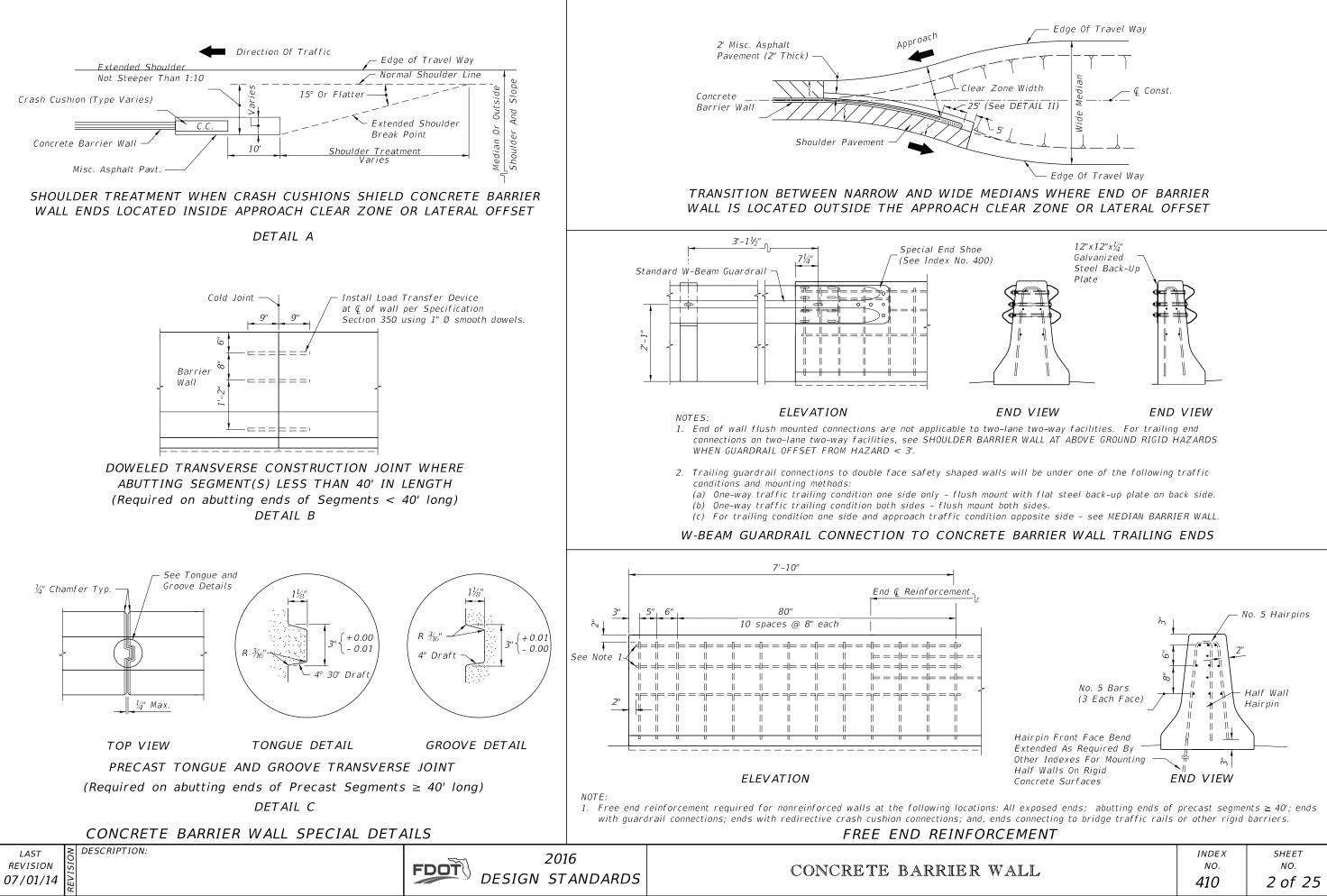


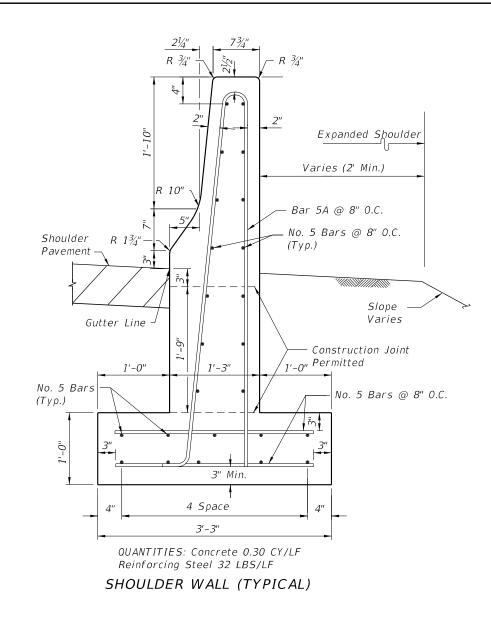
STING	INDEX NO.	SHEET NO.
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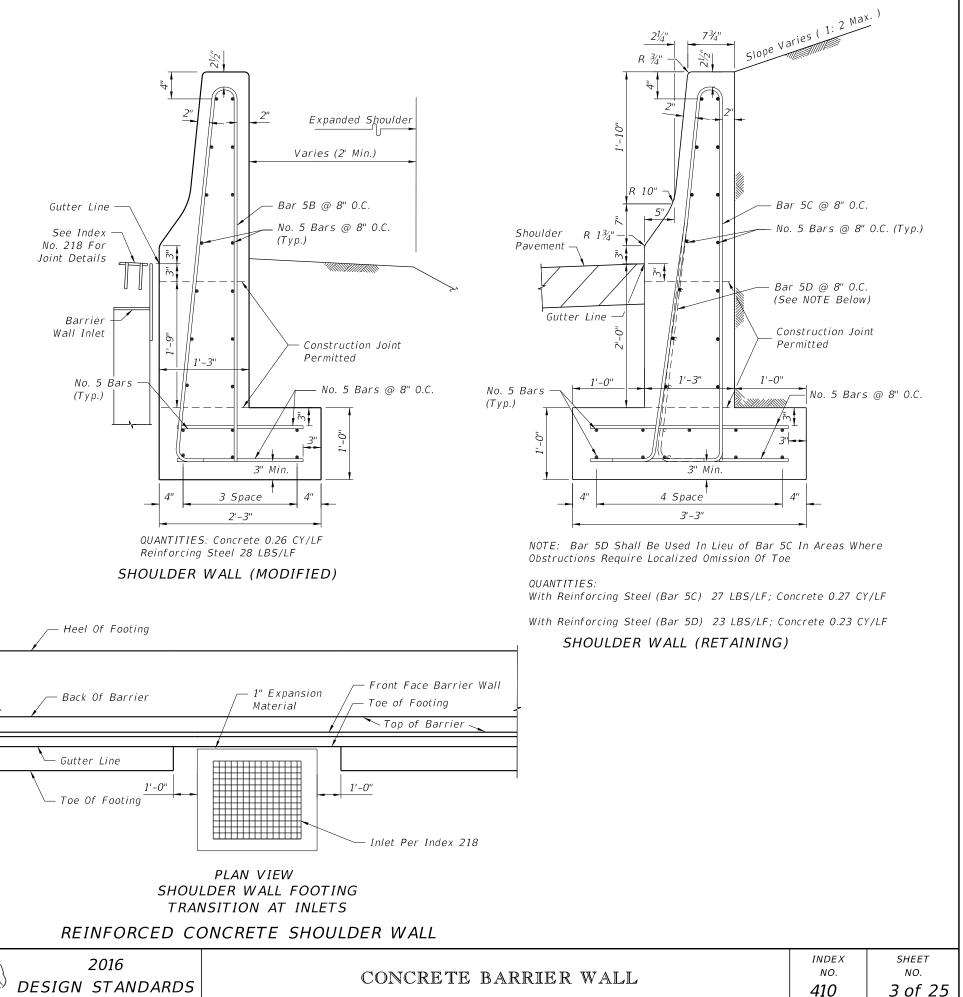


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ISTING	INDEX NO.	SHEET NO.
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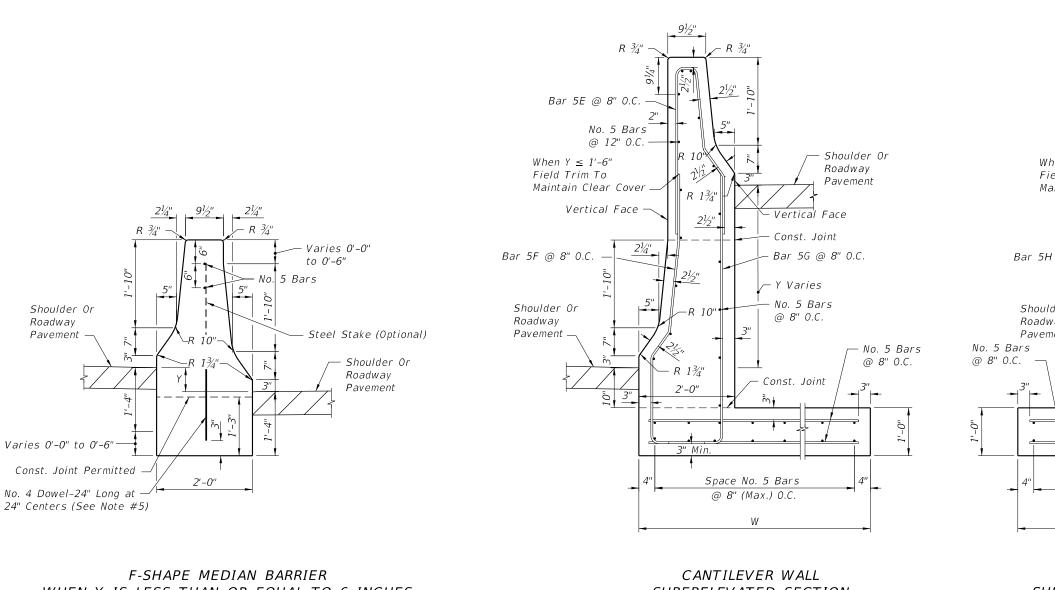
NOTES:

- 1. Reduce the vertical steel spacing to 4 inches 0.C. a distance of 4 feet for each side of all cold or expansion joints.
- 2. Unless otherwise noted, Minimum Segment Wall Length is 20 LF.
- 3. All walls may be made up of segments 20' or more in length provided the segments are joined by a transverse joint in accordance with the CONCRETE BARRIER WALL SPECIAL DETAILS, DETAIL B.
- 4. Quantities shown are for information only. Barrier wall inlets (Index 218) shall be isolated from the barrier wall stem and footing by 1" expansion material.
- 5. All longitudinal reinforcement to be continuous or spliced No. 5 bars. Lap splices a minimum of 2'-0".
- 6. For additional information on Bars 5A, 5B, 5C and 5D, see BAR BENDING DIAGRAMS.

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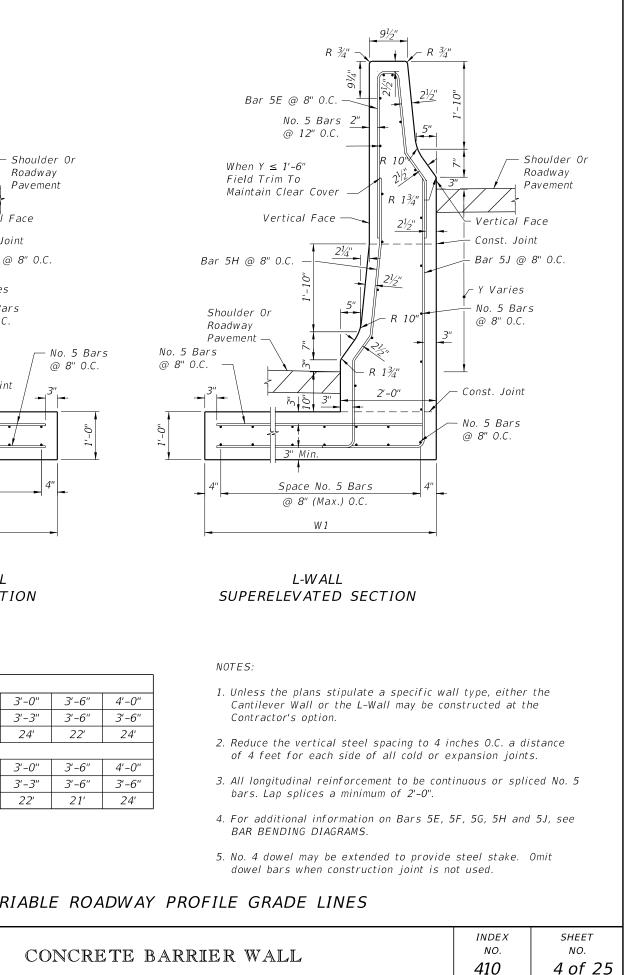


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DIMENSIONS TABLE								
Cantilever	Height Y	1'-0''	1'-6"	2'-0"	2'-6"	3'-0"	3'-6"	4'-0''
Wall	Width W	2'-6"	2'-9"	3'-0"	3'-3"	3'-3"	3'-6"	3'-6"
Min. Segment	29'	27'	25'	23'	24'	22'	24'	
L-Wall	Height Y	1'-0"	1'-6"	2'-0"	2'-6"	3'-0"	3'-6"	4'-0''
	Width W1	2'-6"	2'-9"	3'-0"	3'-3"	3'-3"	3'-6"	3'-6"
Min. Segment	Wall Length	26'	24'	22'	21'	22'	21'	24'

## MEDIAN BARRIER WALL FOR SUPERELEVATED SECTIONS WITH VARIABLE ROADWAY PROFILE GRADE LINES



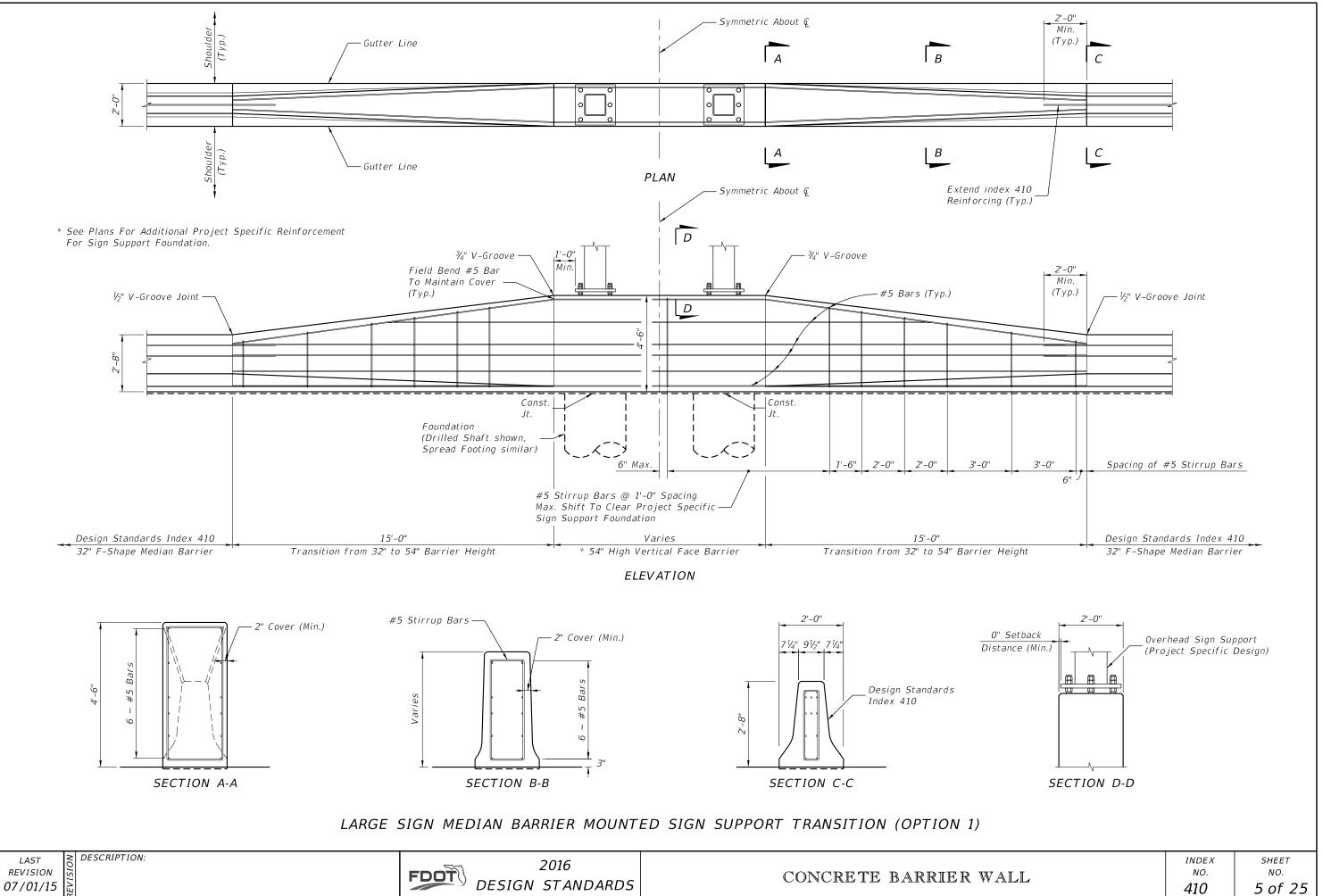
WHEN Y IS LESS THAN OR EQUAL TO 6 INCHES

# SUPERELEVATED SECTION

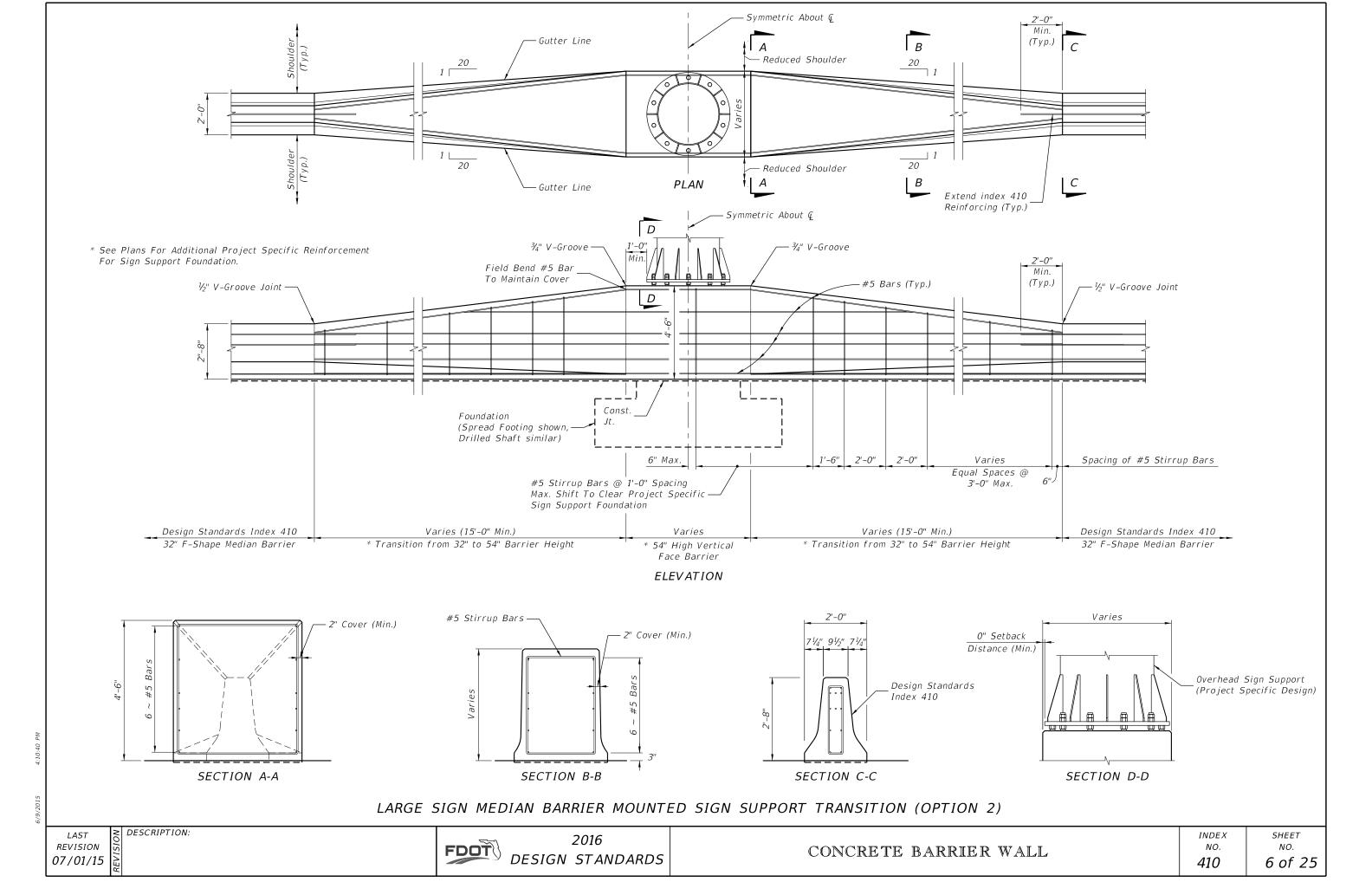
L-wan	Width W1	2'-6"	2'-9''	3'-0''	3'-3''	3'-3"	
n. Segment	Wall Length	26'	24'	22'	21'	22'	

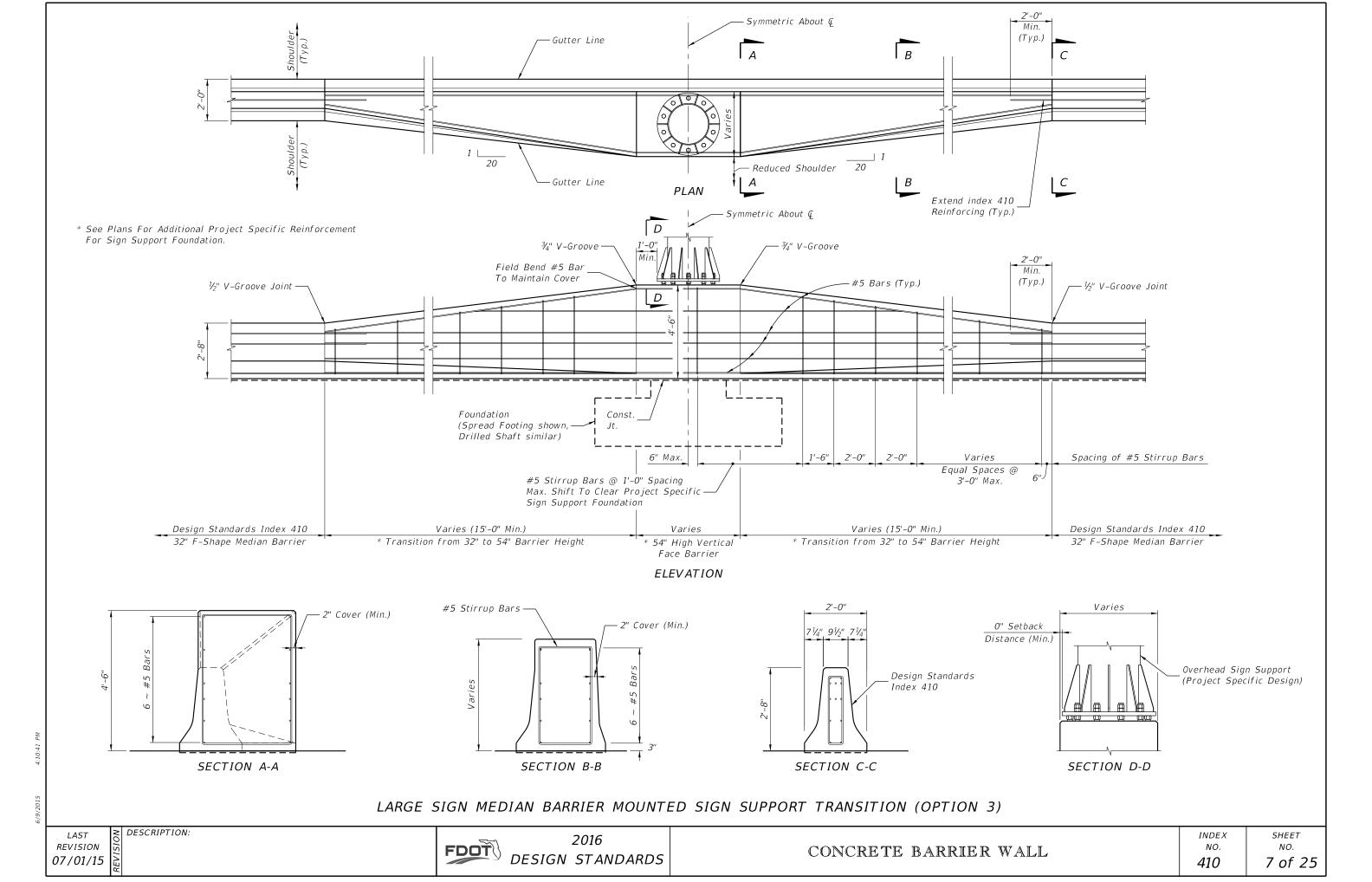
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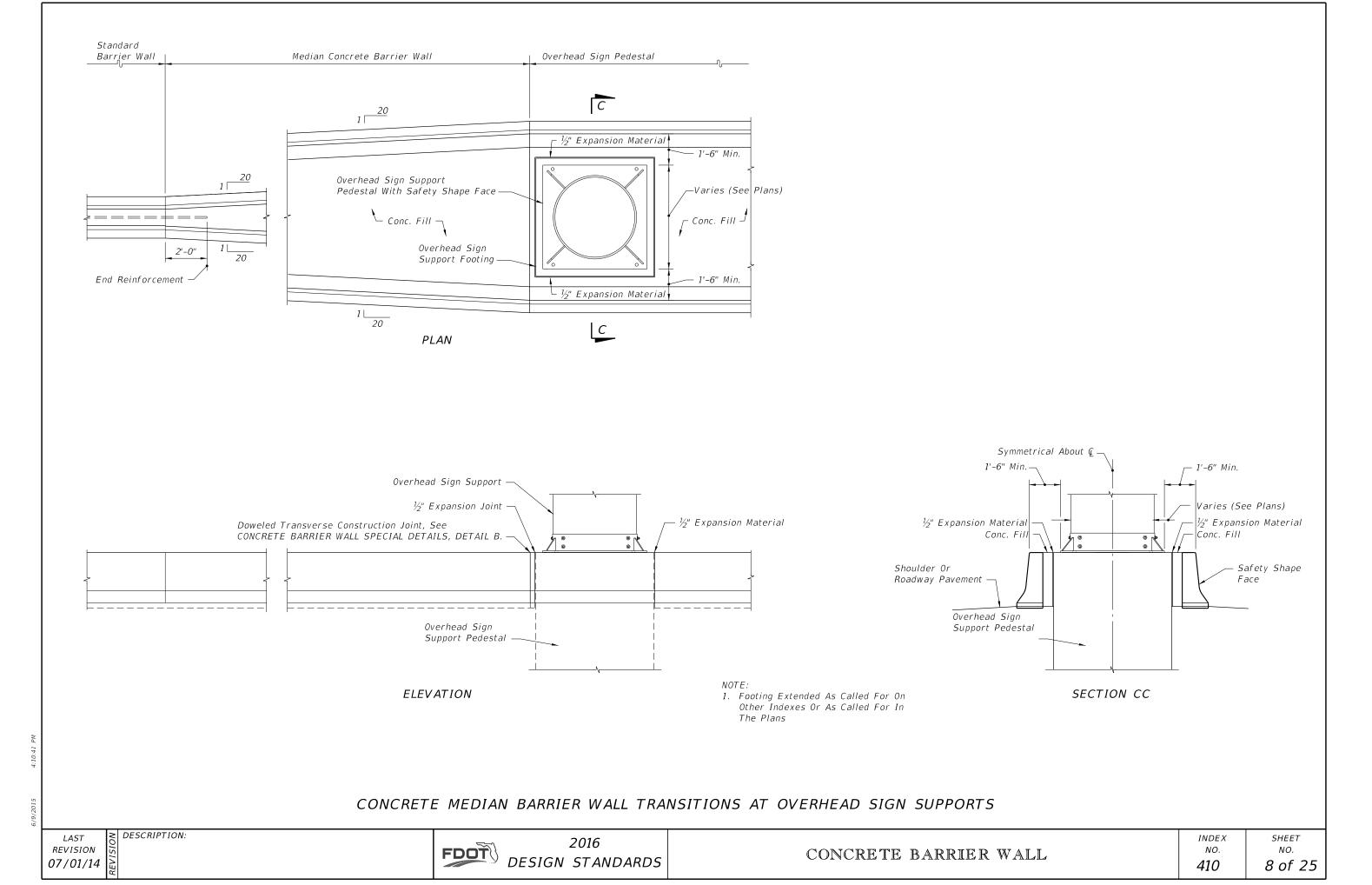
2016 FDOT DESIGN STANDARDS

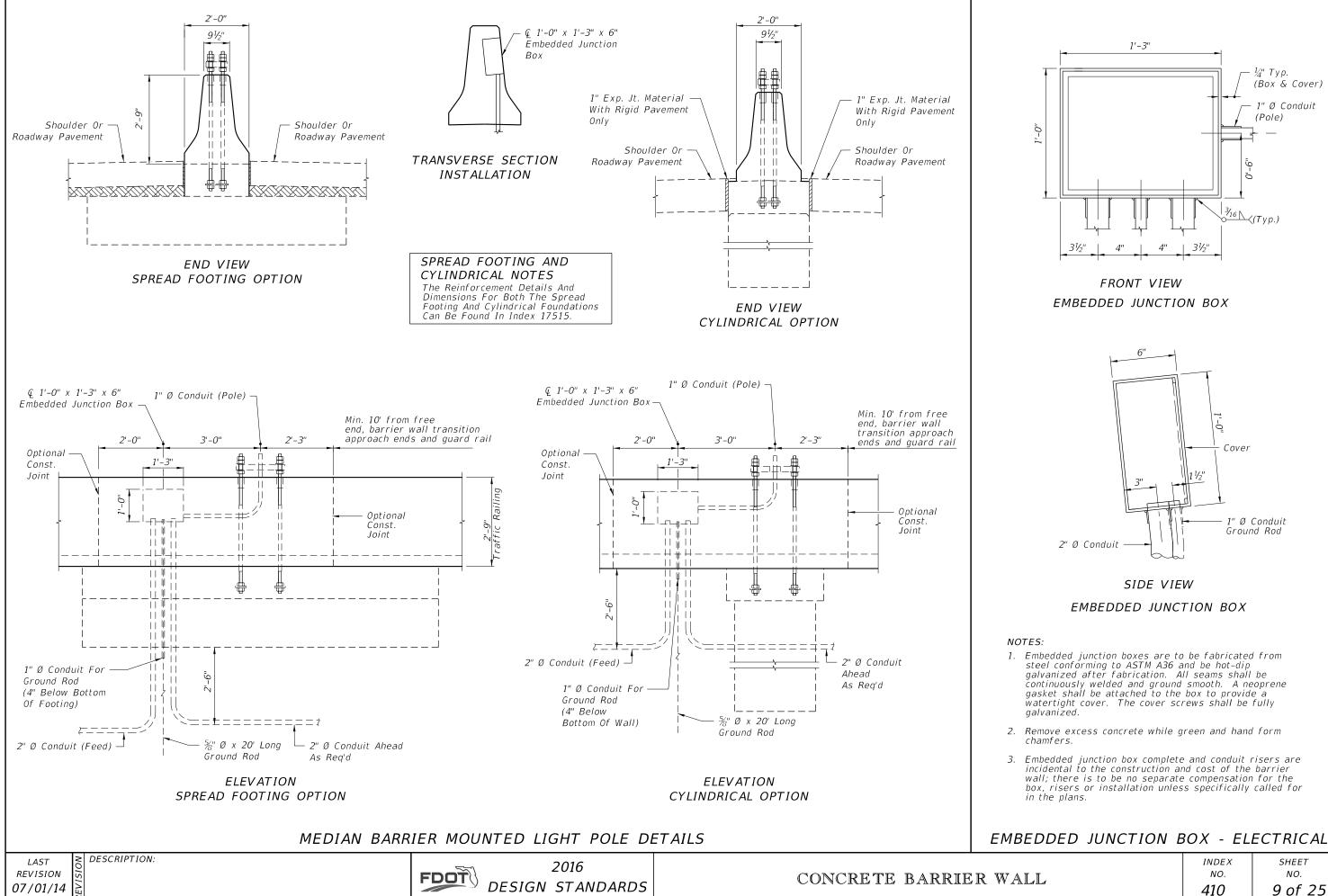


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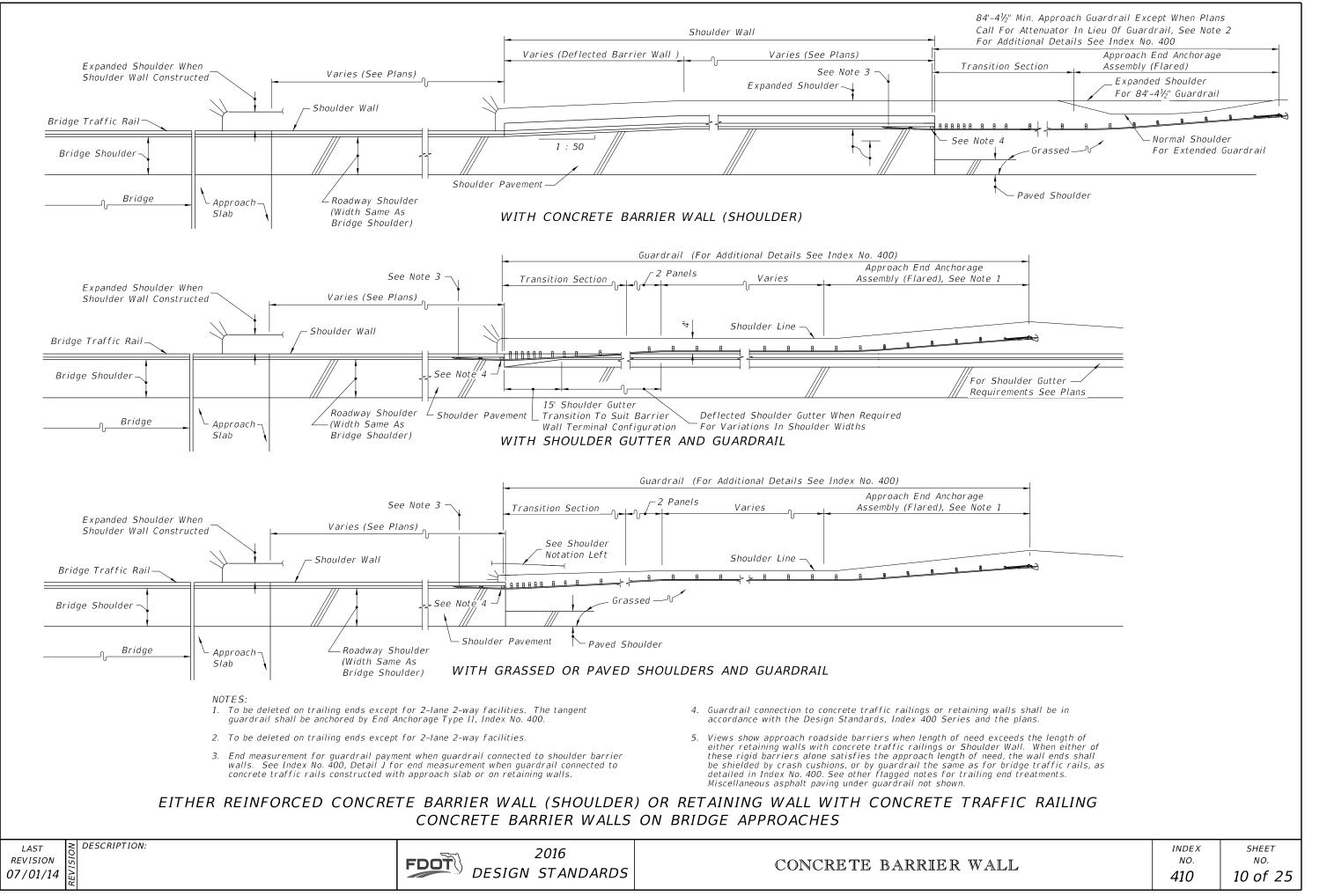


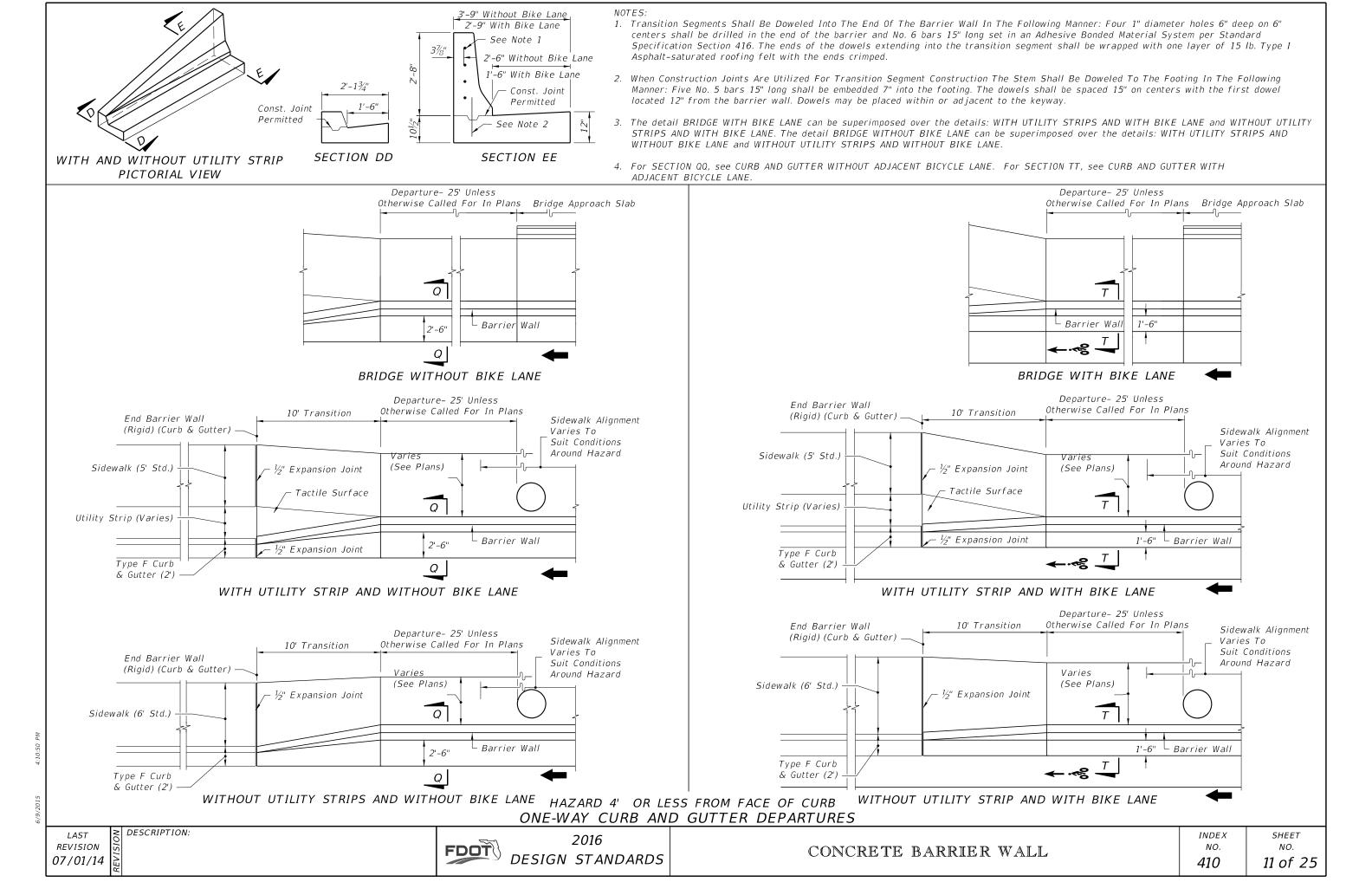


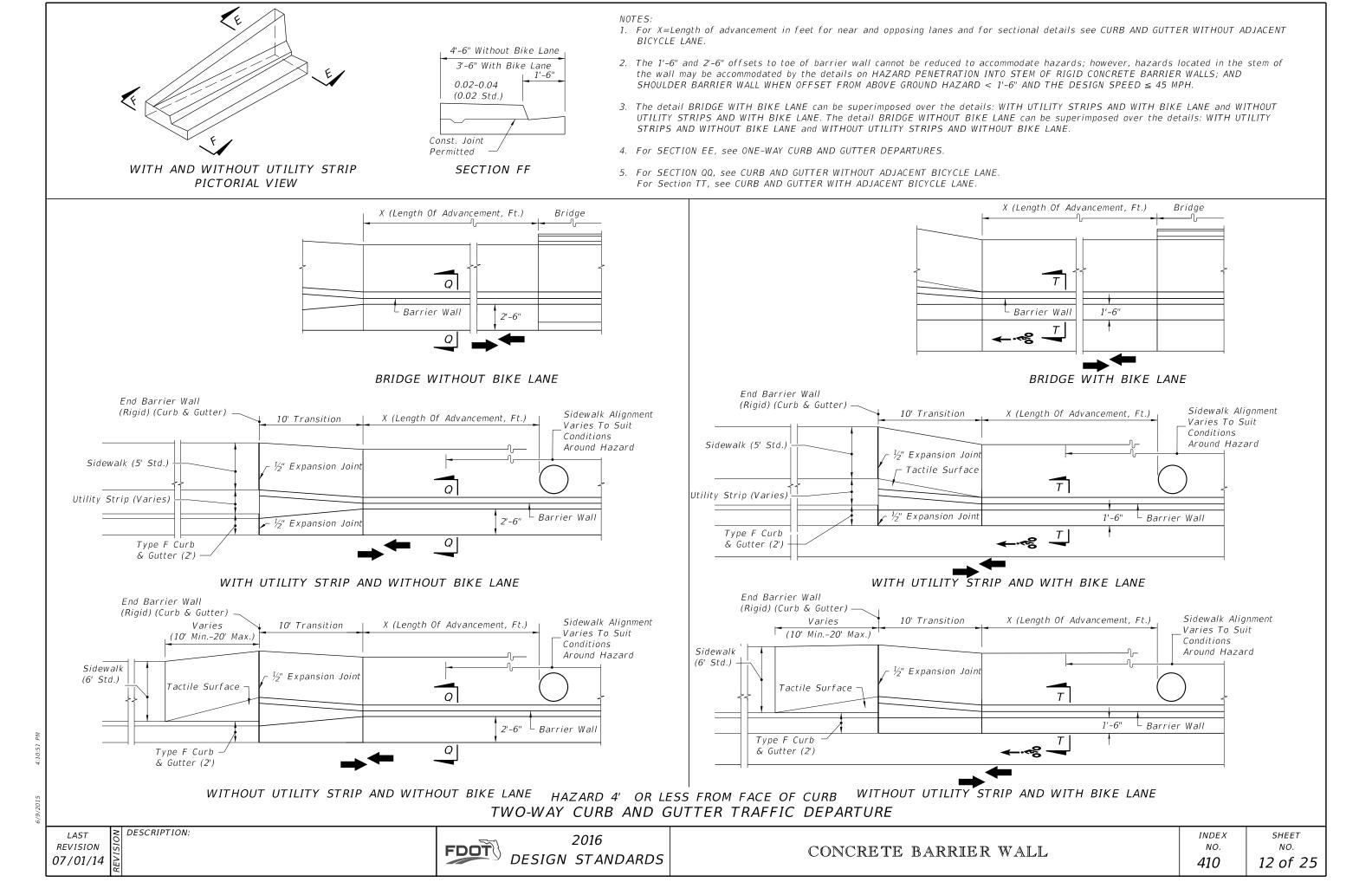


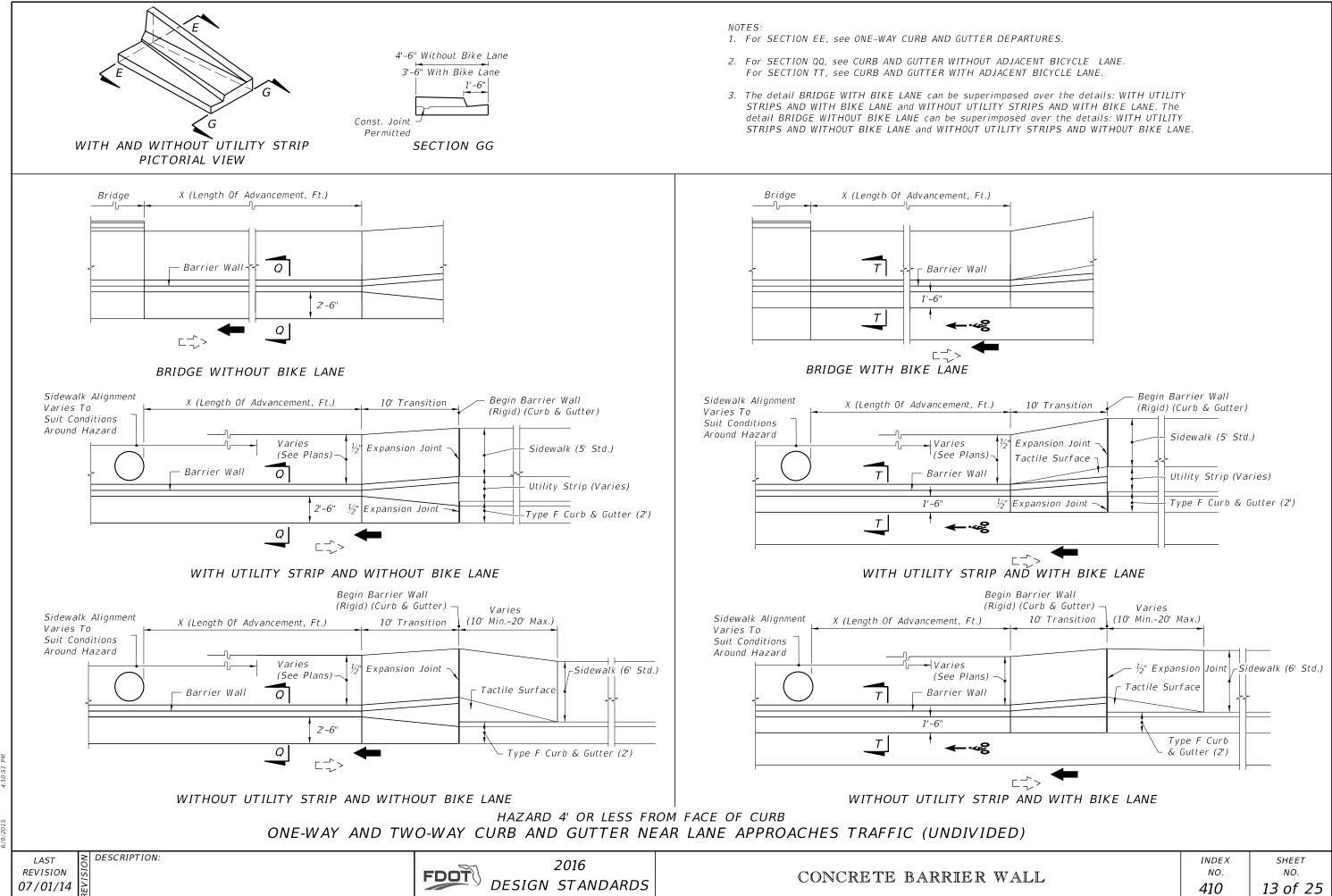
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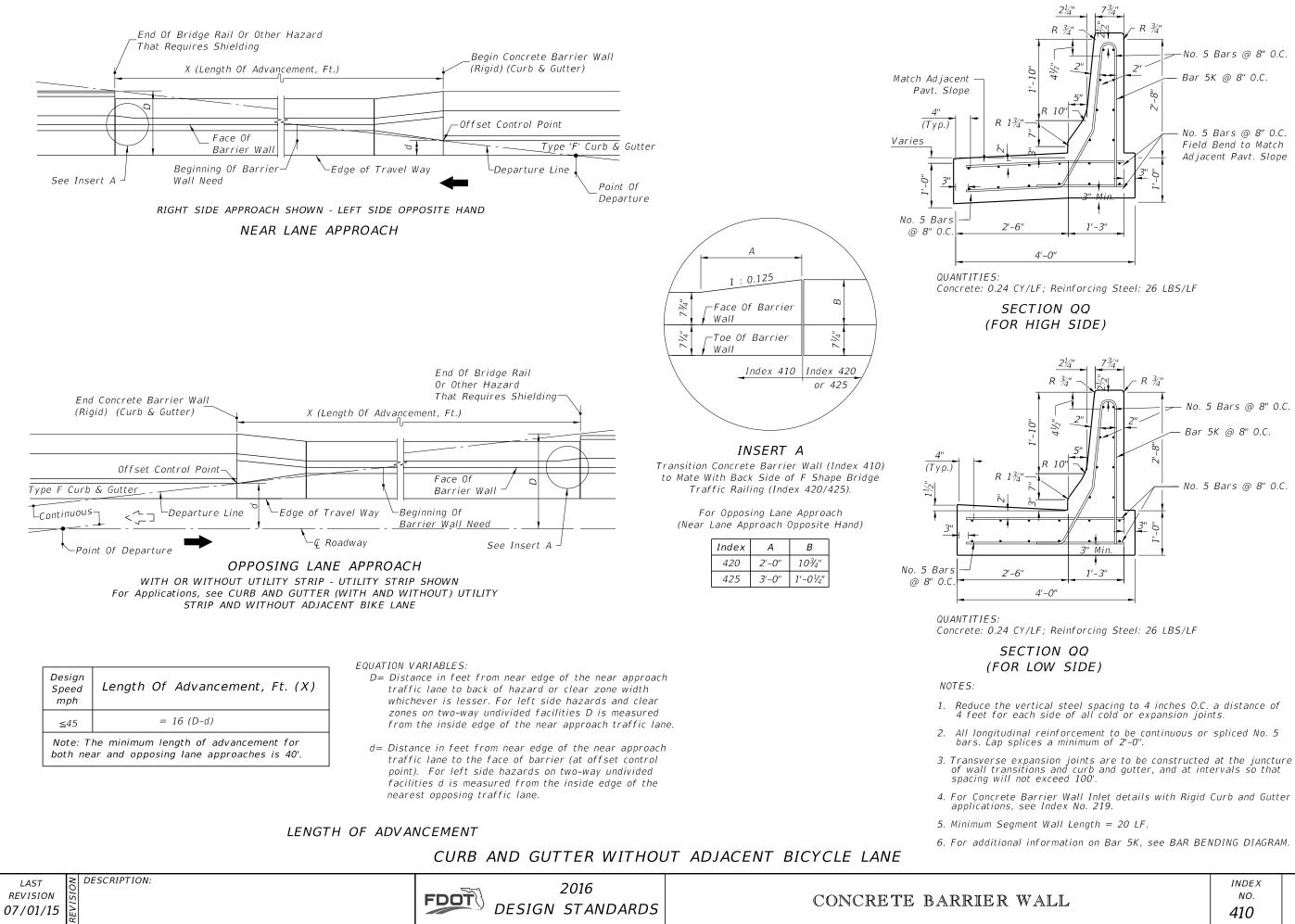
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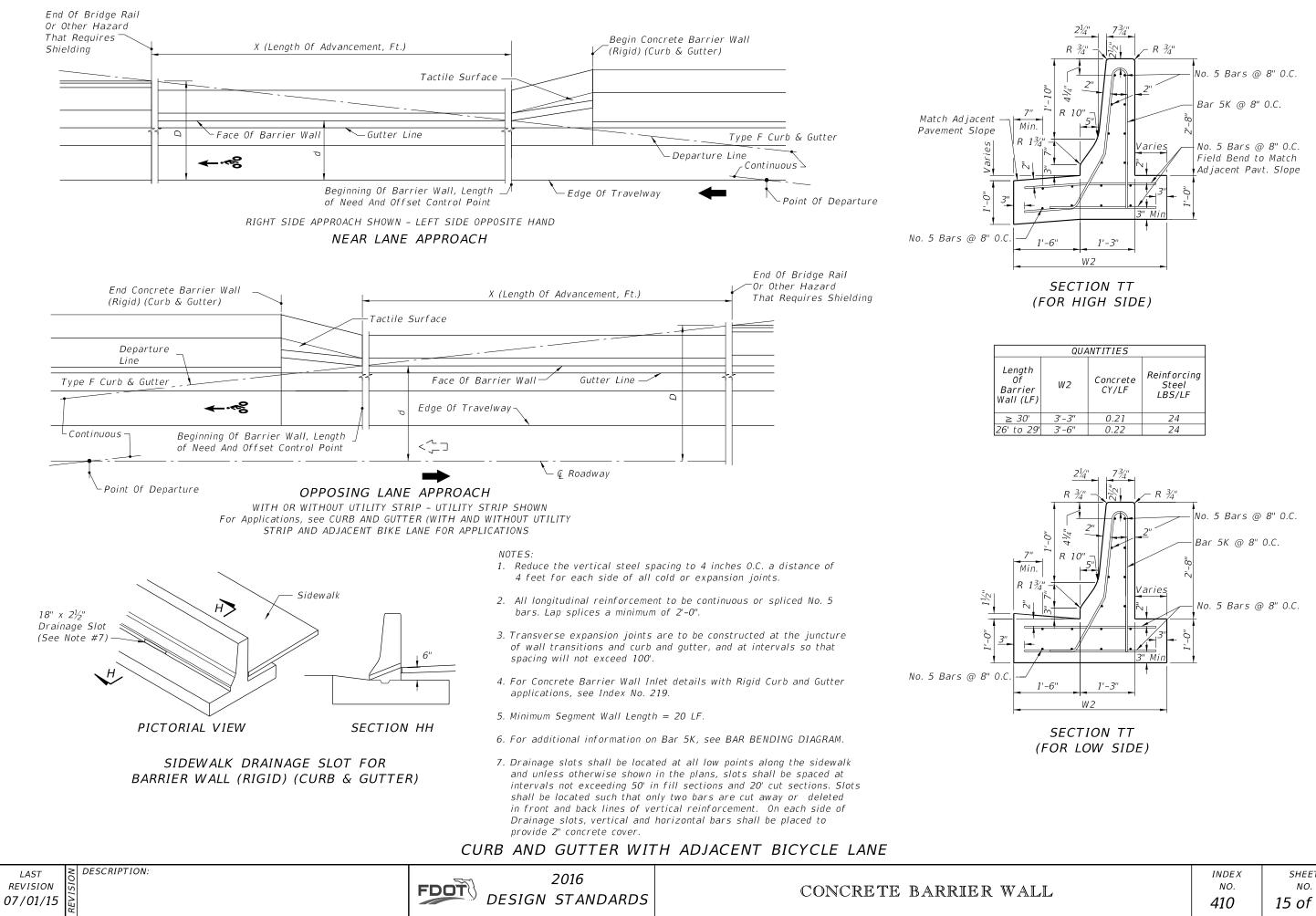








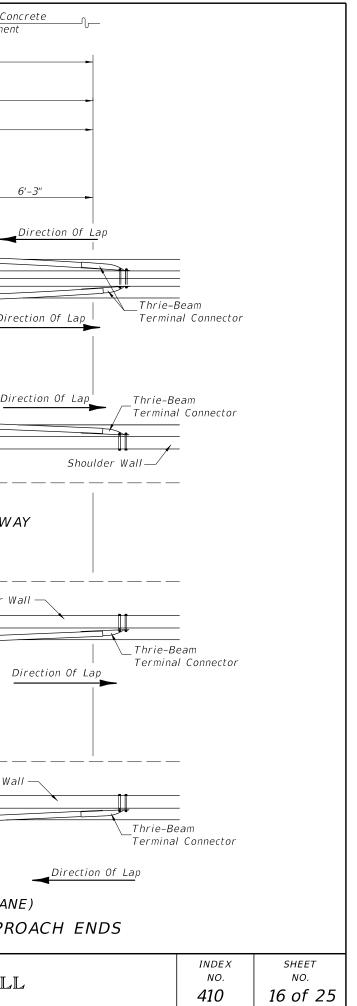
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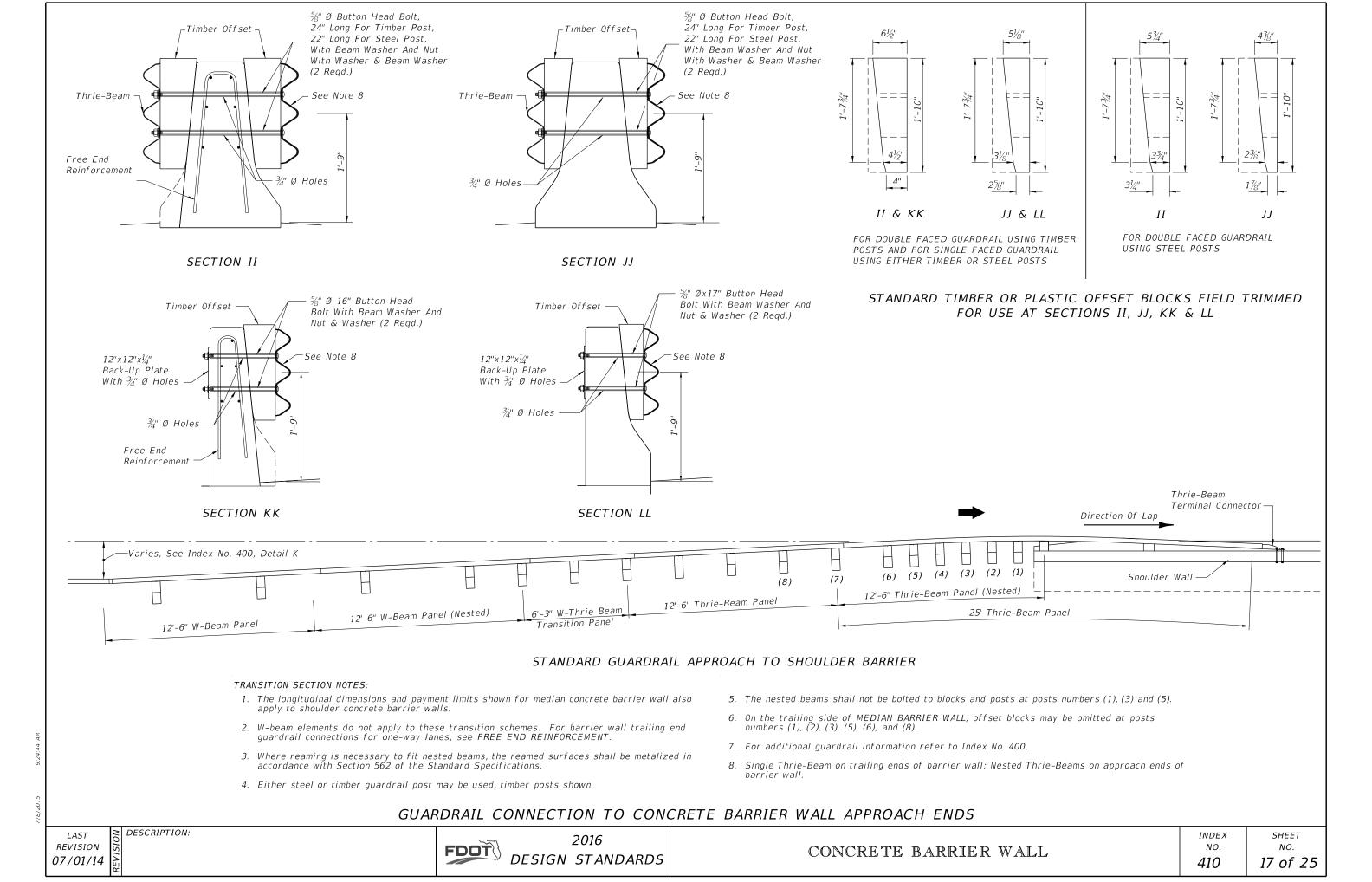


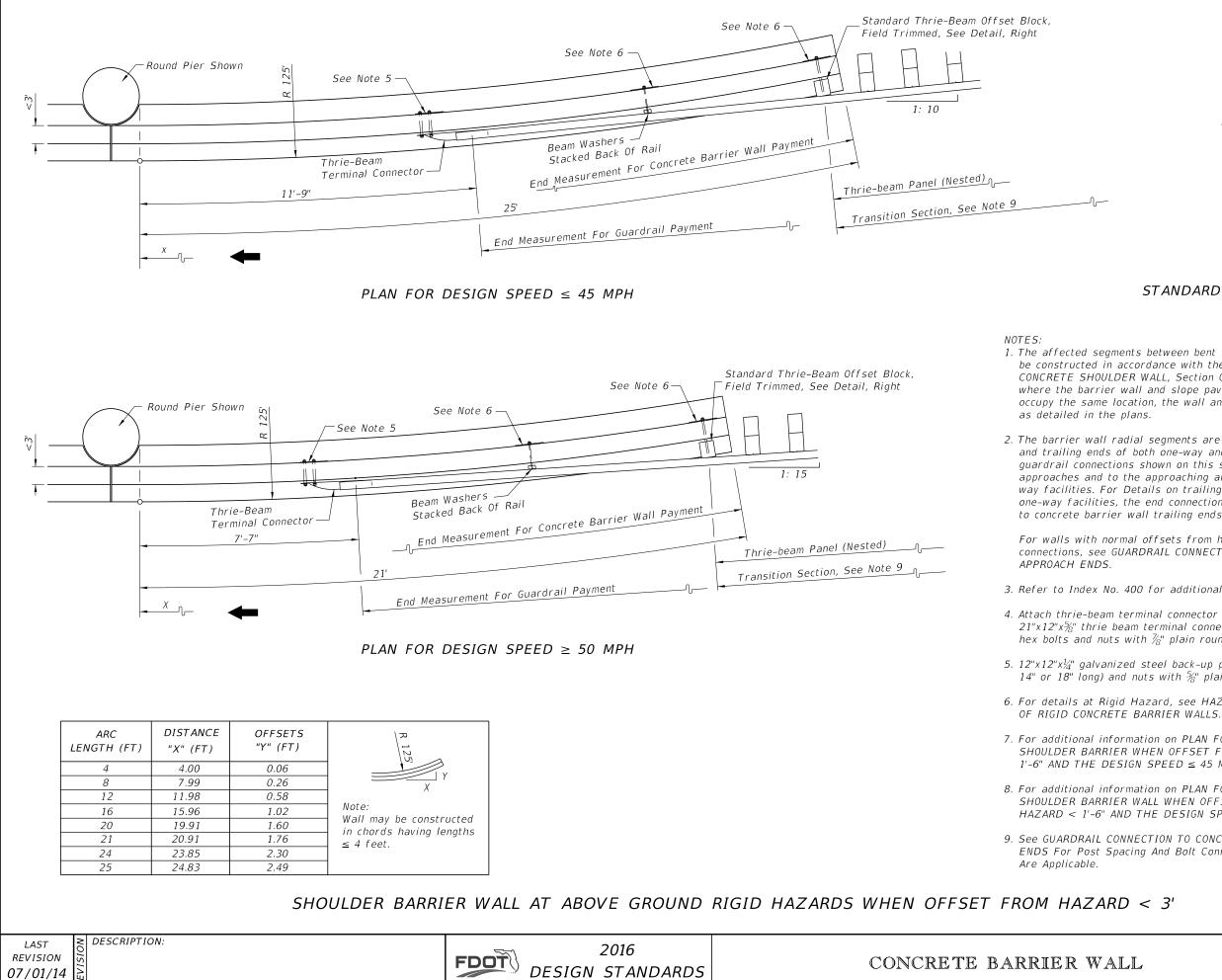
QUANTITIES									
W2	Concrete CY/LF	Reinforcing Steel LBS/LF							
3'-3''	0.21	24							
3'-6"	0.22	24							

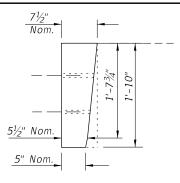
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										-\		E	nd Meası	irement I	For Gua	ardrail Pa	ayment	
	-								Transition	Section, see	e TRANSITI	ON SECT	TION NOT	ES				
			Double Face	ed Guardr		-3" W-Thrie Transition		12'-6	" Thrie-Bea	m Panel					25' Th	nrie-Beam	n Panel	
											F		6" Thrie- ic Appro			Wall		
	-	12	-6" W-Beam Pa	anel					-	3@3	3'-1½"			6 @ 1'-6	3/" /4		6'-3	
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			Single Fac	ced Guardr	ail						MED	IAN B.	ARRIEF	R WALL	-			
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			Single Fac	ced Guardr	ail		RIGHT	SIDL A	AF F NOAC	II ONL-W	AT LAN	LJOF	NIGH	וסוכ ו		NUACI	1 1 W O-L	ANL TWO-M
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										LEFT	SIDE A	PPRO	асн о	NE-WA	Y LAI	NES		
			Single Fac	ced Guardı	ail											<b>.</b>		-
	-	12'-6" V	N-Beam Panel (	(Nested)													K 	<i>L</i>
																		Shoulder W
NOTES:	I																	
1. For Section Guardrail a	nd Offse	t Block View	vs, see wa	all with 5-	%"x15" la	ong HS hex	nector to me c bolts and i	nuts with	er					<b></b>			K	L
ST ANDARD SHOULDER			Att	tach to sh	oulder b	arrier wall	heads and n ' with a 21"> plate and 5-	x12"x5/;"									DOACU	
			Ion	ng HS hex	bolts ar	nd nuts wit s and nuts.	'h %" plain r	ound	GUARE									OR FAR LA VALL APPI
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FOR USE WITH EITHER 1: 10 OR 1: 15 GUARDRAIL TRANSITIONS

## STANDARD THRIE-BEAM OFFSET BLOCK (FIELD TRIMMED)

1. The affected segments between bent supports or pier columns shall be constructed in accordance with the detail for REINFORCED CONCRETE SHOULDER WALL, Section QQ, or Section TT. In cases where the barrier wall and slope pavement or other structure would occupy the same location, the wall and structure are to be modified

2. The barrier wall radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane twoway facilities. For Details on trailing ends of two-way multilane and one-way facilities, the end connection on W-Beam guardrail connection to concrete barrier wall trailing ends may be used.

For walls with normal offsets from hazards and their guardrail connections, see GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL

3. Refer to Index No. 400 for additional guardrail information.

4. Attach thrie-beam terminal connector to shoulder barrier wall with a  $21"x12"x\frac{5}{8}"$  thrie beam terminal connector plate and  $5-\frac{7}{8}"x12"$  long HS hex bolts and nuts with  $\frac{7}{8}$ " plain round washers under heads and nuts.

5. 12"x12"x<sup>1</sup>/<sub>4</sub>" galvanized steel back-up plate with <sup>5</sup>/<sub>8</sub>" post bolts (either 14" or 18" long) and nuts with  $\frac{5}{2}$ " plain round washers under nuts.

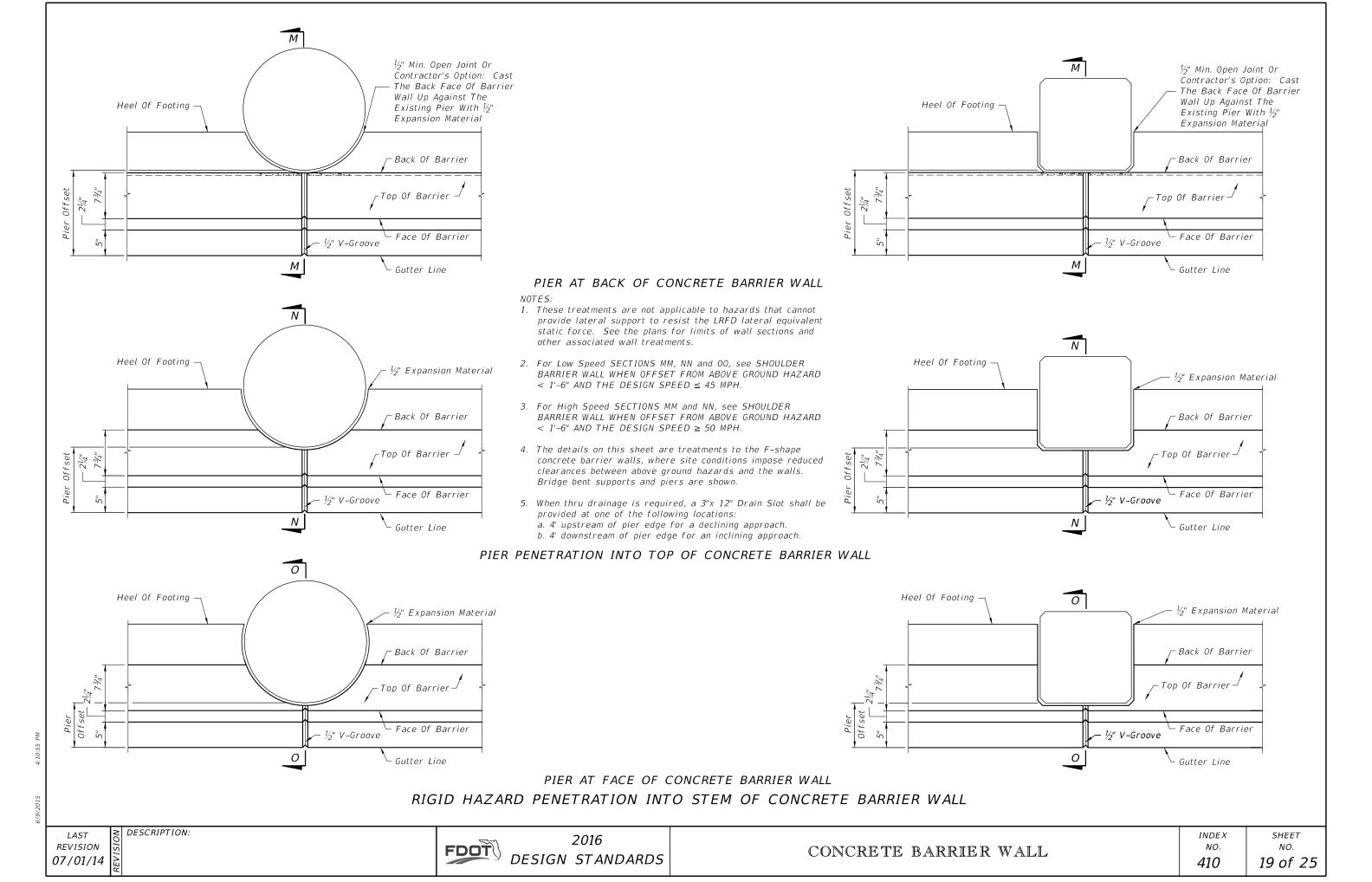
6. For details at Rigid Hazard, see HAZARD PENETRATION INTO STEM

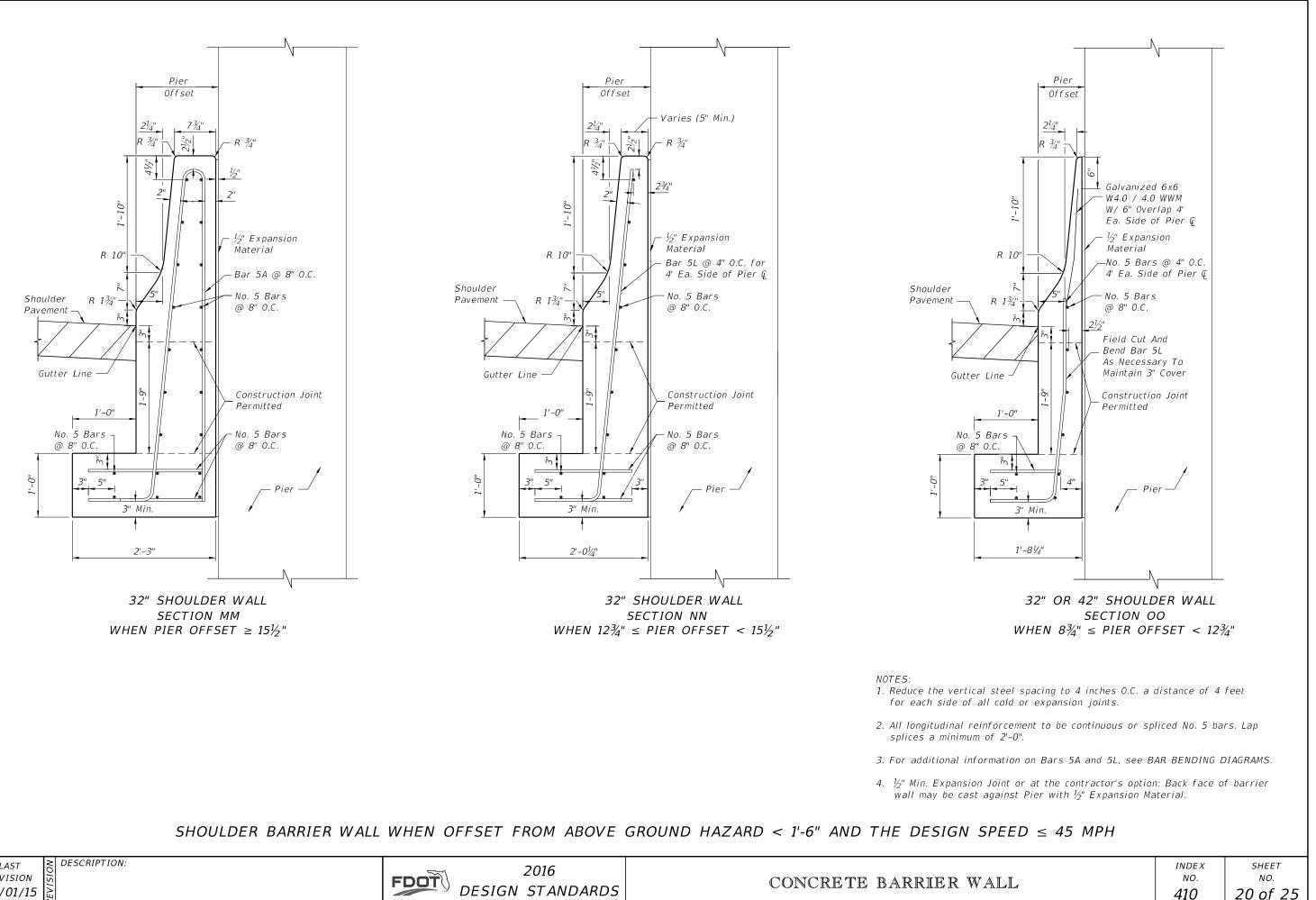
7. For additional information on PLAN FOR DESIGN SPEED  $\leq$  45 MPH, see SHOULDER BARRIER WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6" AND THE DESIGN SPEED ≤ 45 MPH.

8. For additional information on PLAN FOR DESIGN SPEED  $\geq$  50 MPH, see SHOULDER BARRIER WALL WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6'' AND THE DESIGN SPEED  $\geq$  50 MPH.

9. See GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL APPROACH ENDS For Post Spacing And Bolt Connections, Steel Or Timber Posts

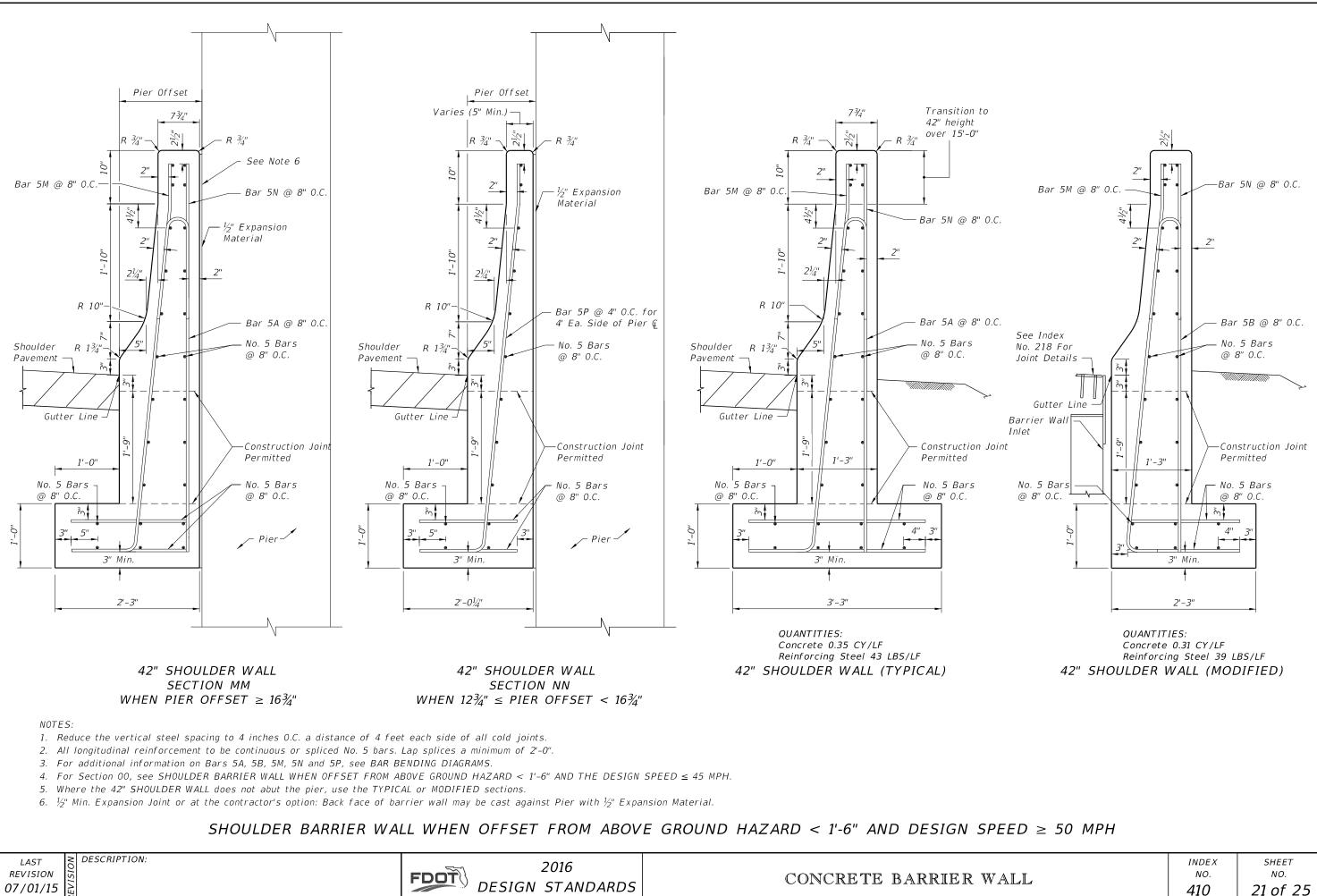
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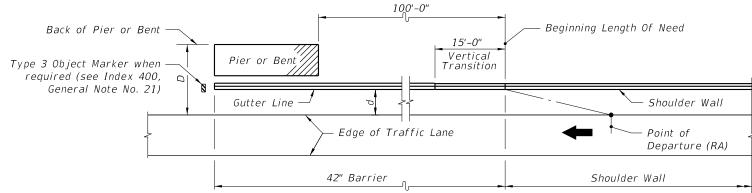


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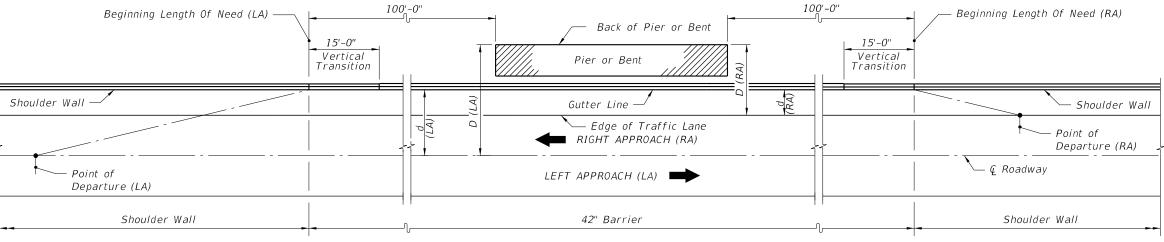
DESIGN STANDARDS



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TWO-LANE TWO-WAY TRAFFIC

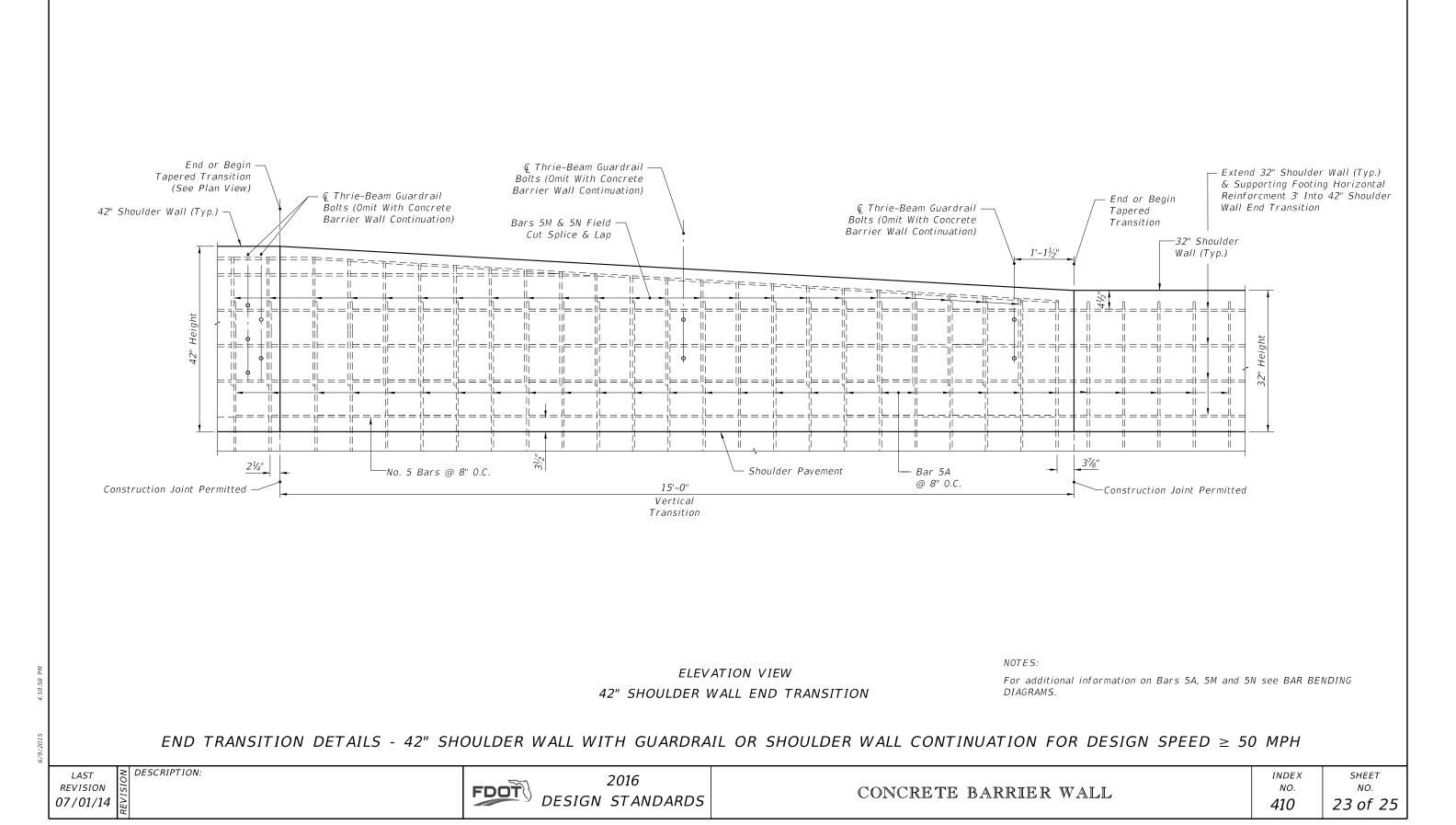
SHOULDER BARRIER WALL WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6" AND DESIGN SPEED  $\geq$  50 MPH

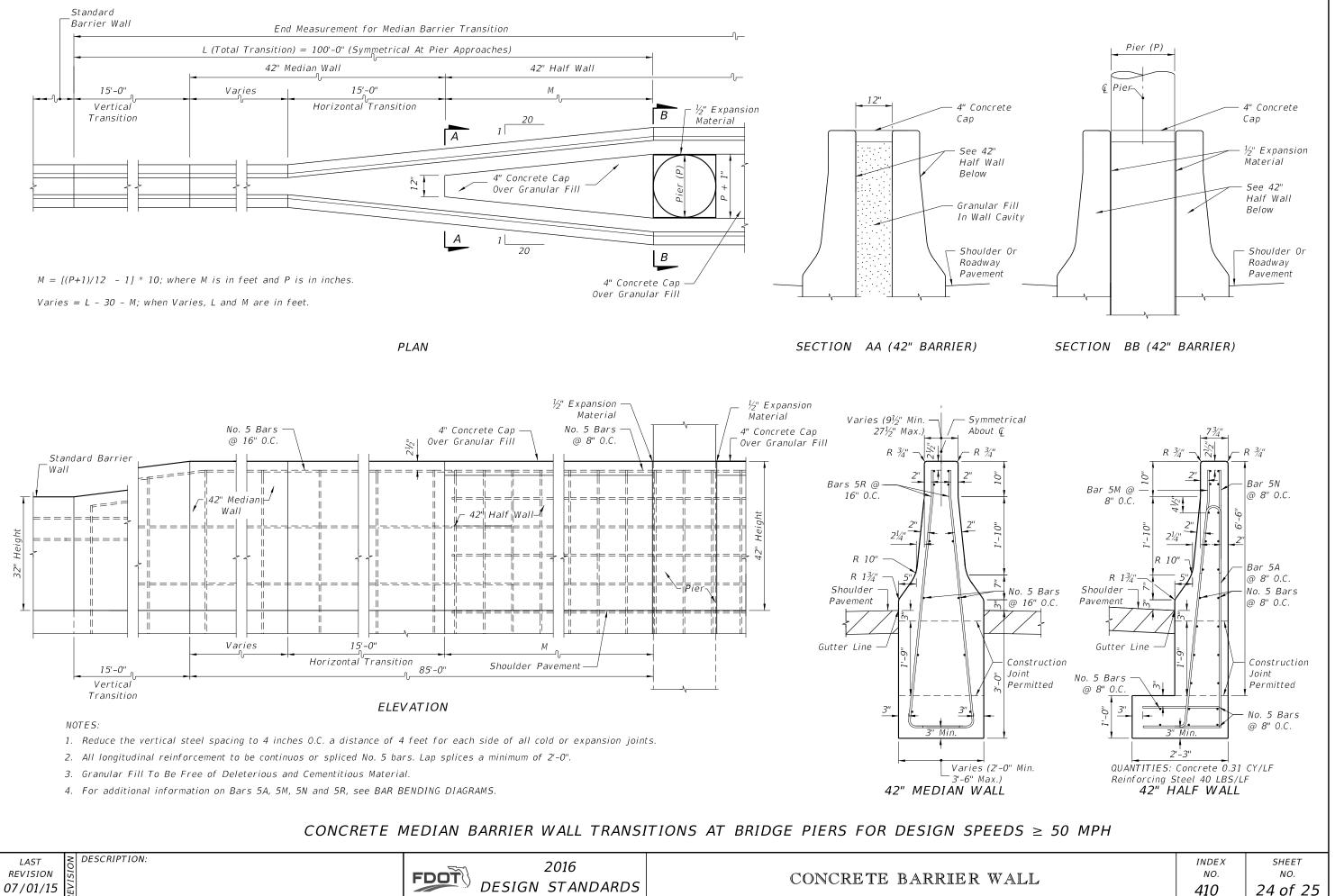
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CONCRETE BARRIER WAL

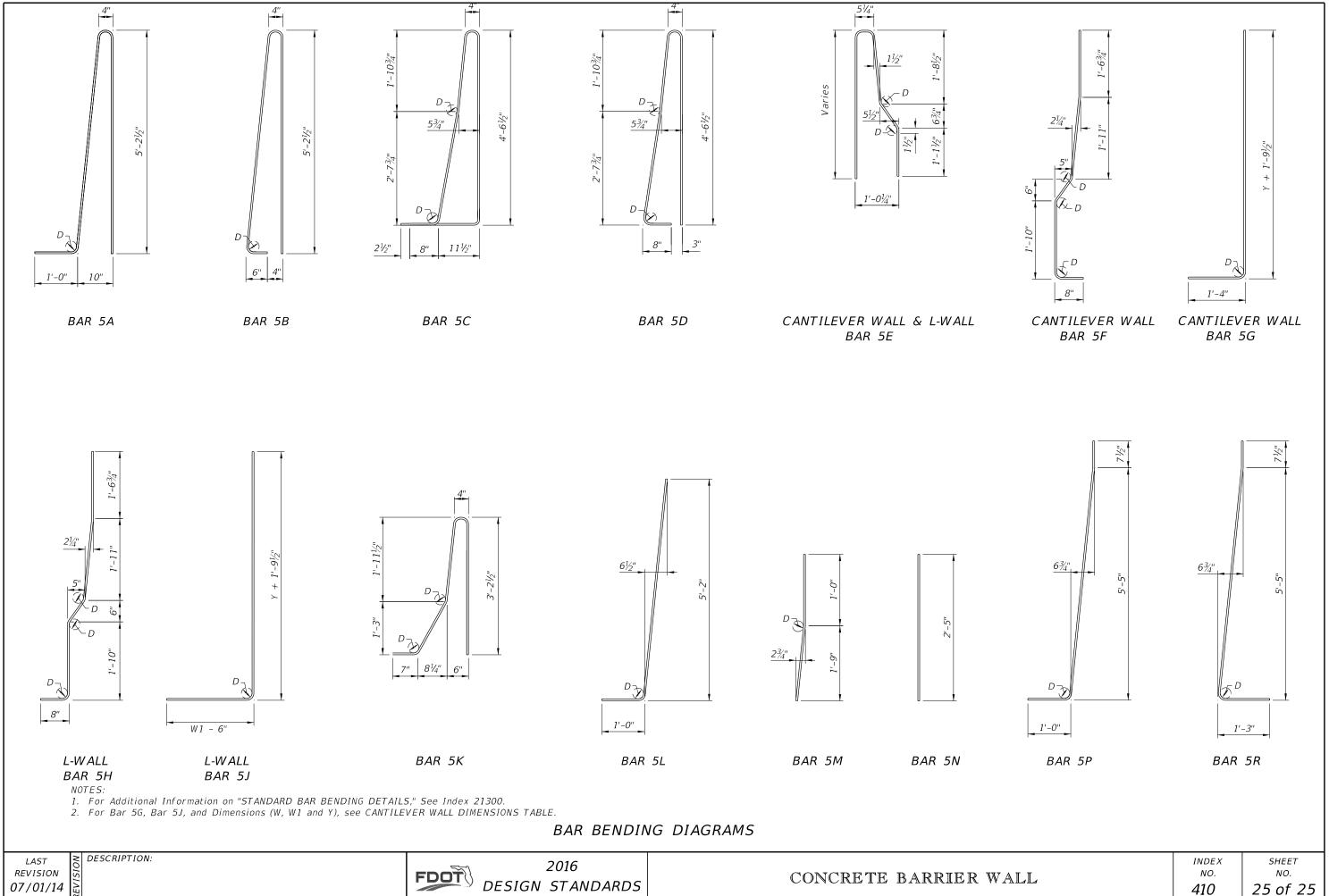
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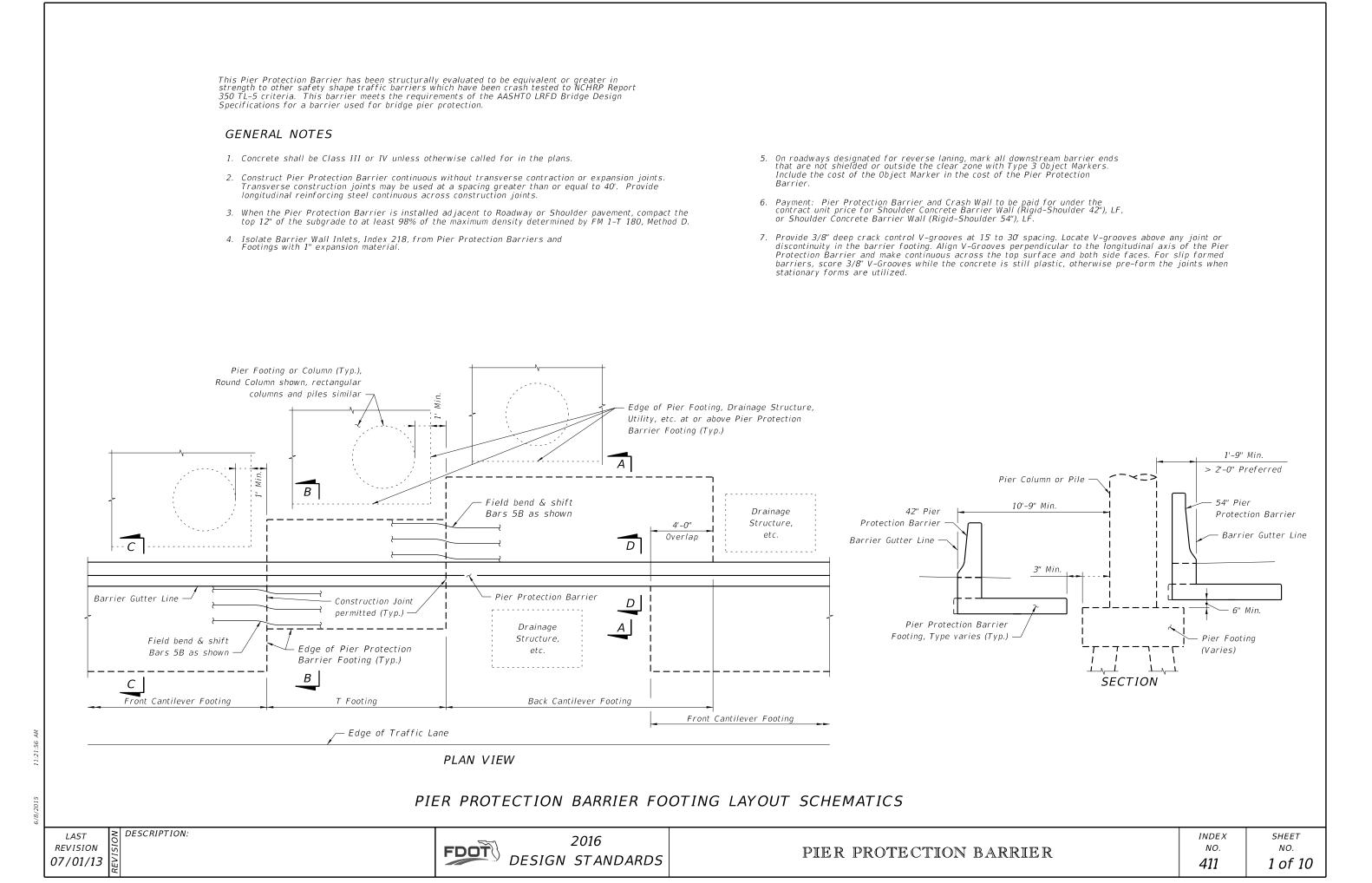


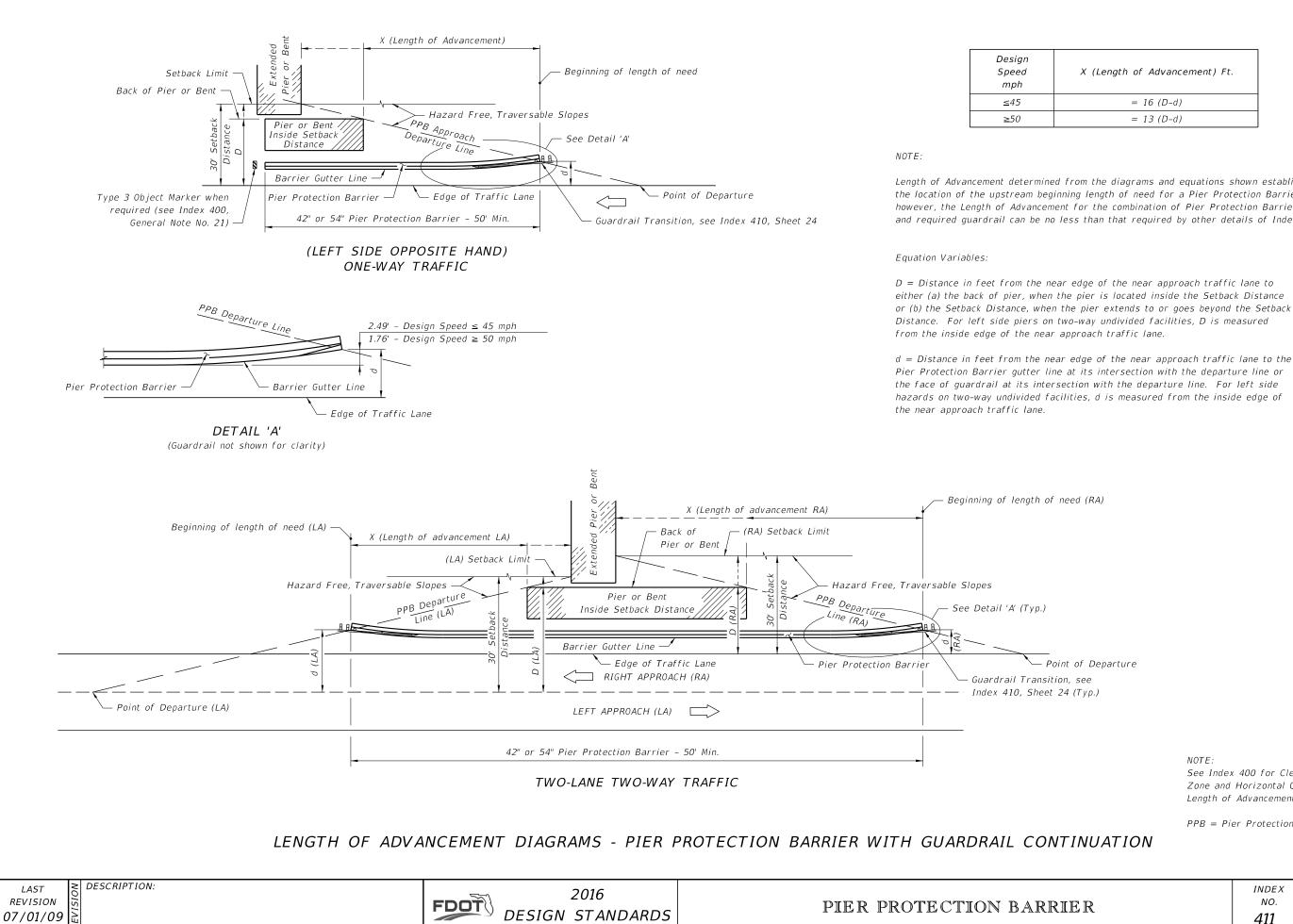


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## X (Length of Advancement) Ft.

= 16 (D-d)
= 13 (D-d)

Length of Advancement determined from the diagrams and equations shown establishes the location of the upstream beginning length of need for a Pier Protection Barrier, however, the Length of Advancement for the combination of Pier Protection Barrier and required guardrail can be no less than that required by other details of Index 400.

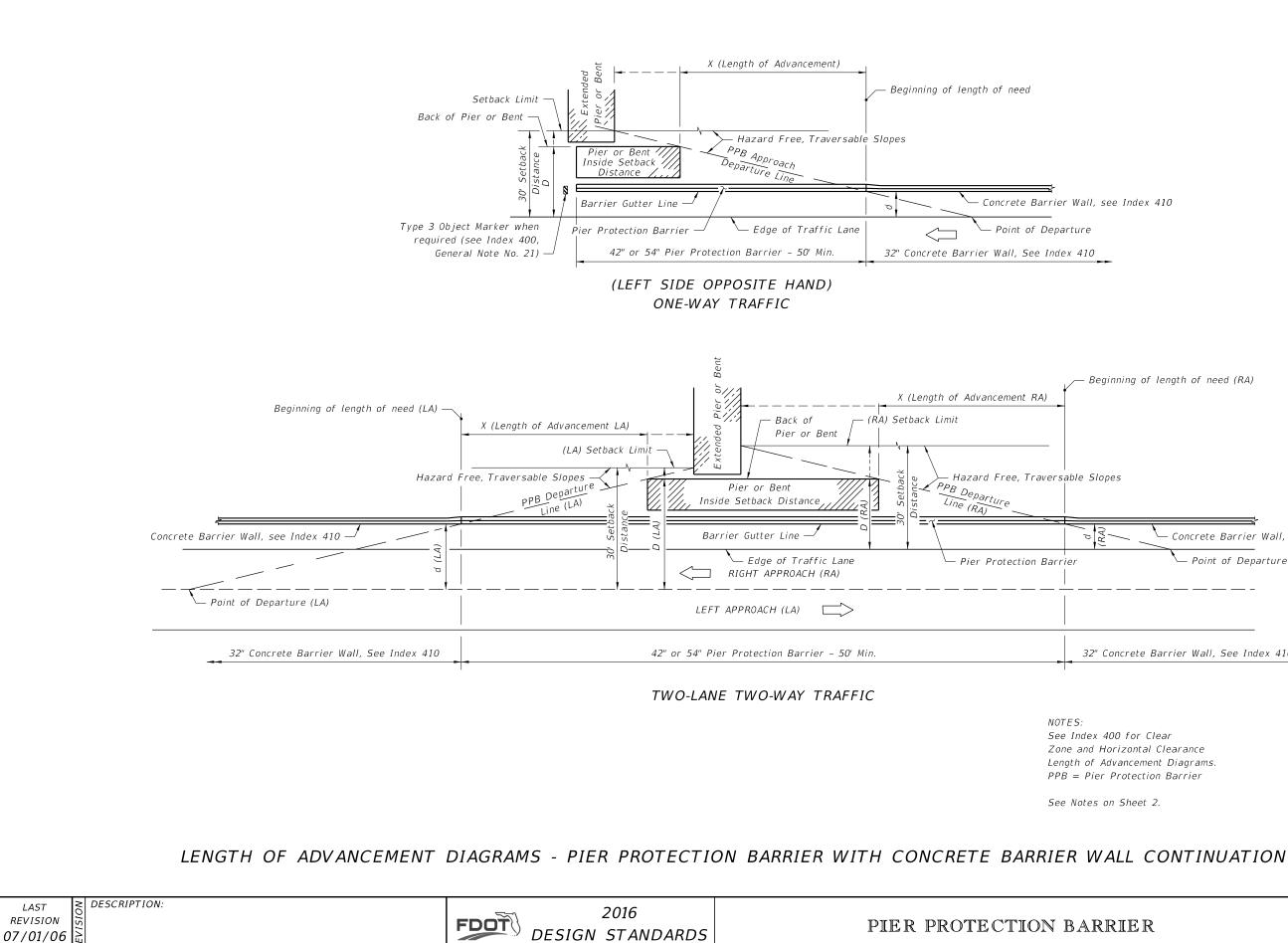
either (a) the back of pier, when the pier is located inside the Setback Distance or (b) the Setback Distance, when the pier extends to or goes beyond the Setback

Pier Protection Barrier gutter line at its intersection with the departure line or the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of

NOTE: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams.

PPB = Pier Protection Barrier

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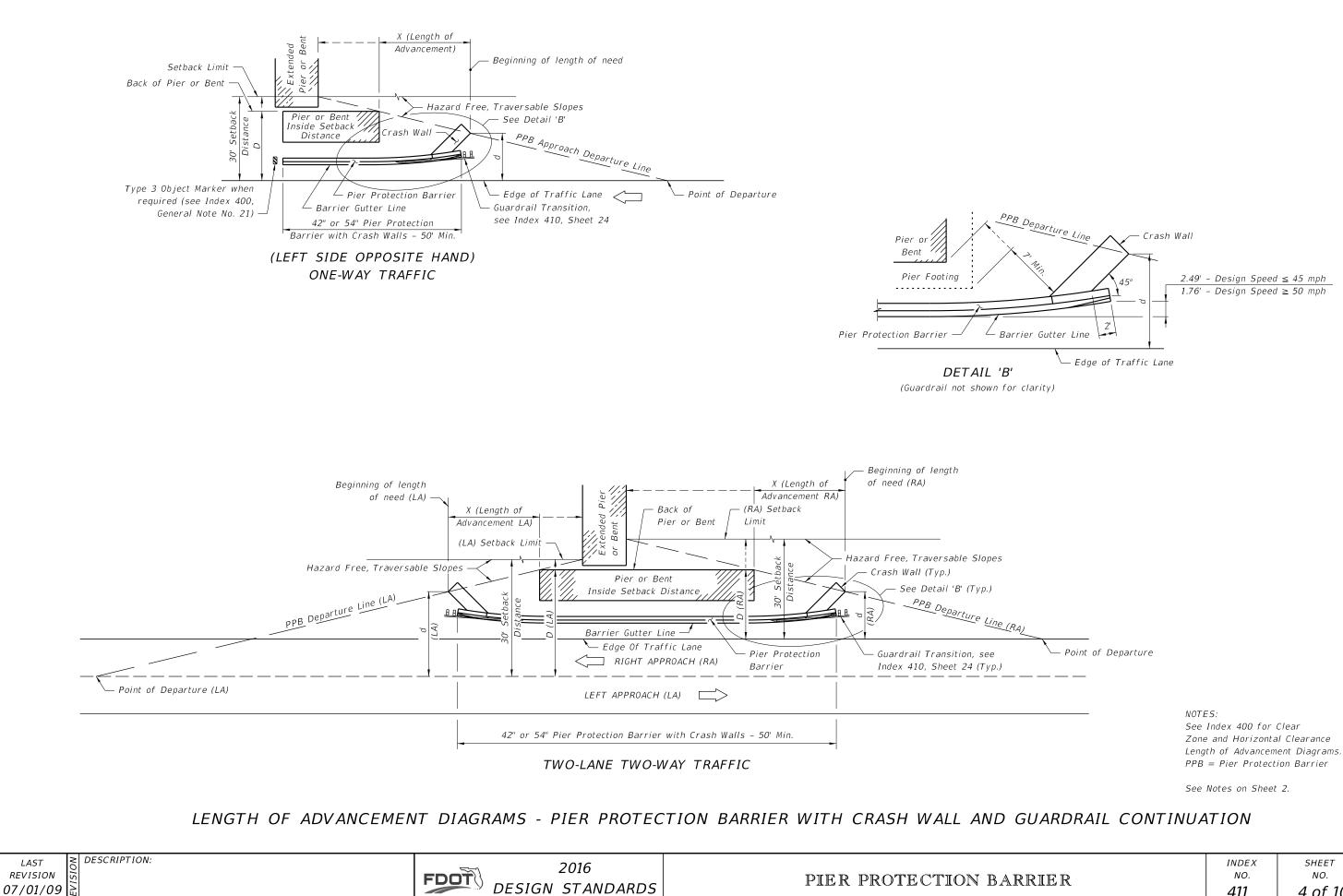
DESIGN STANDARDS

- Beginning of length of need (RA)

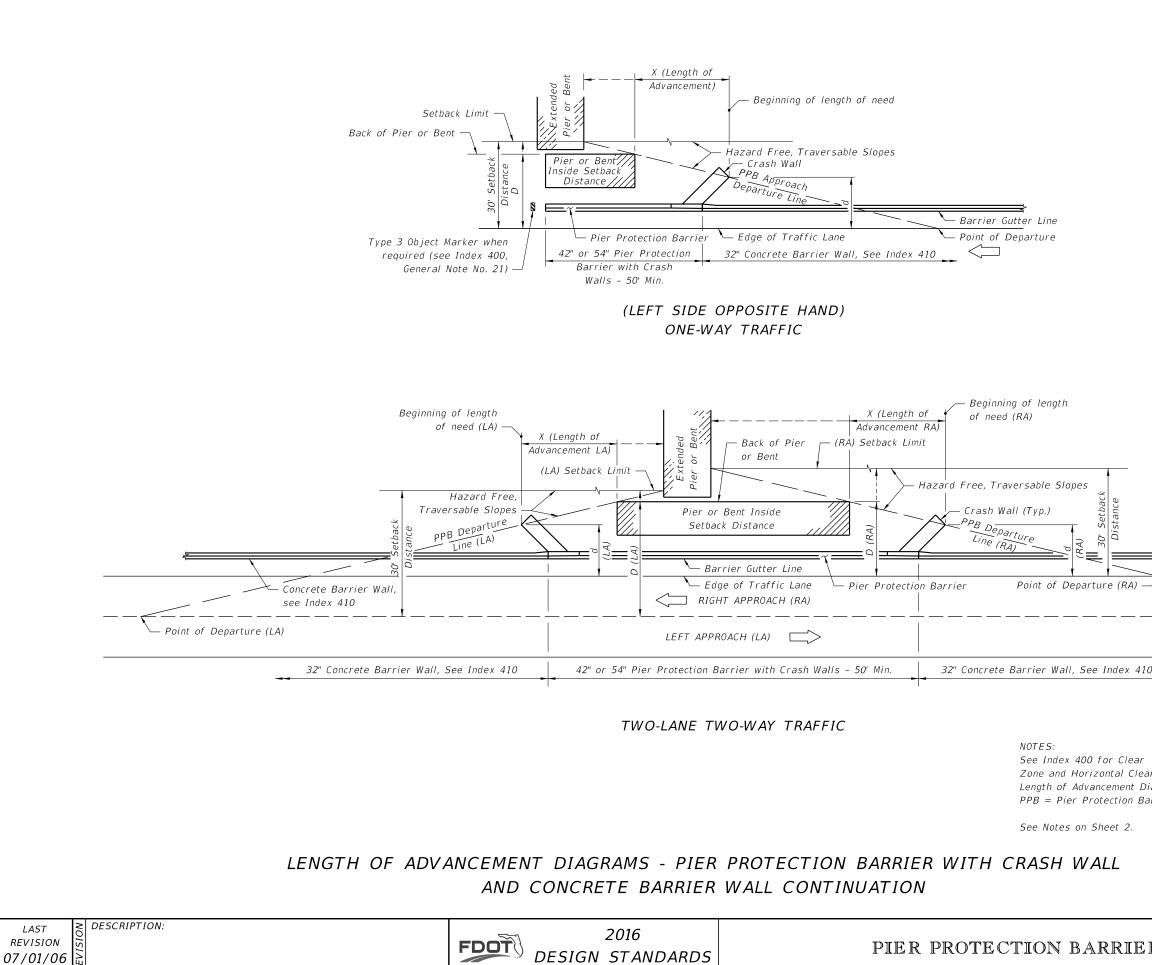
- Concrete Barrier Wall, see Index 410 Point of Departure

32" Concrete Barrier Wall, See Index 410

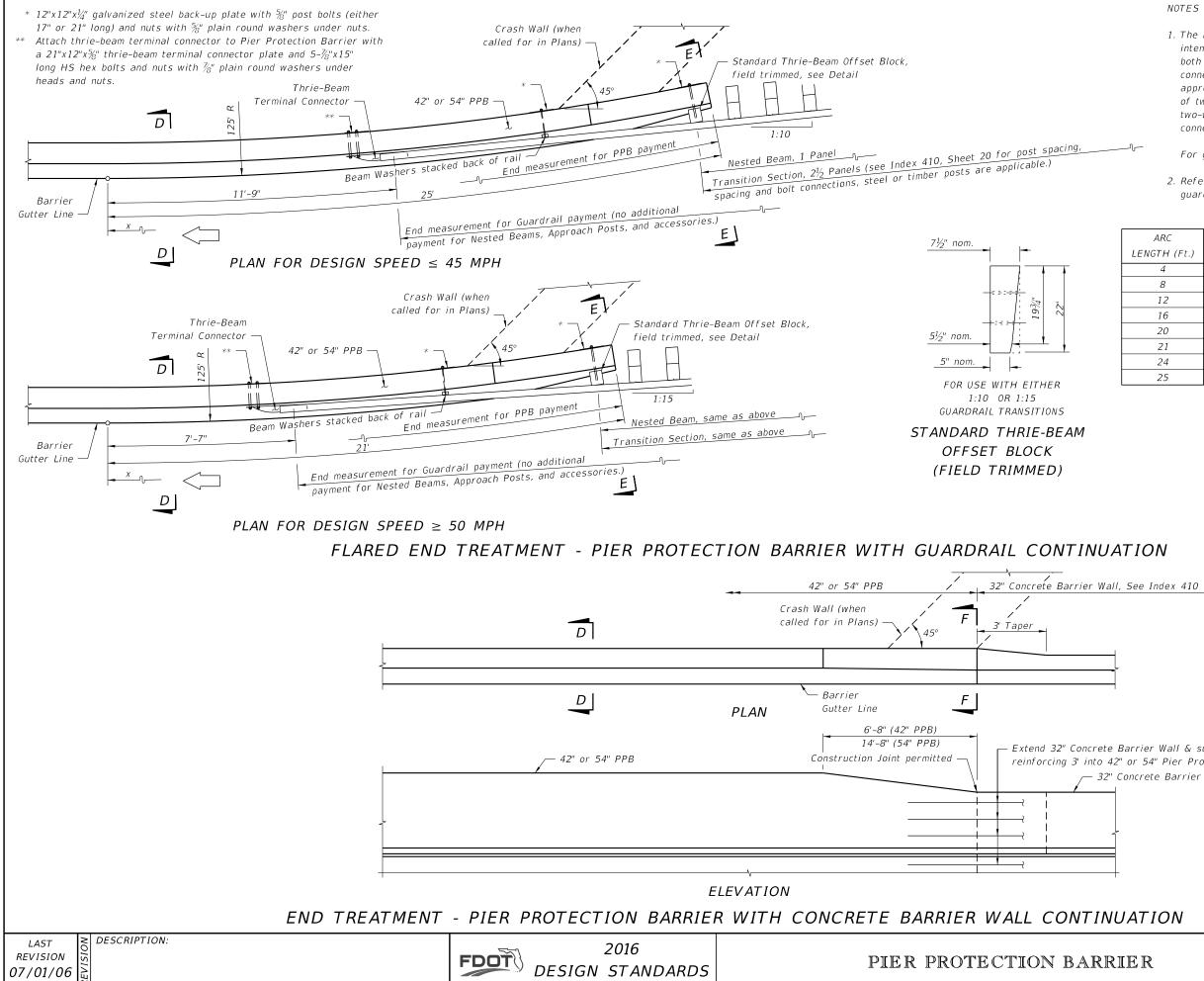
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Concrete Barrier Wall See Index 410	,	
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arance Diagrams. Barrier		
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### NOTES

1. The Pier Protection Barrier radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane two-way facilities. On trailing ends of two-way multilane and one-way facilities the end connection on Index 410, Sheet 2 may be used.

For guardrail connections, see Index 410, Sheet 20.

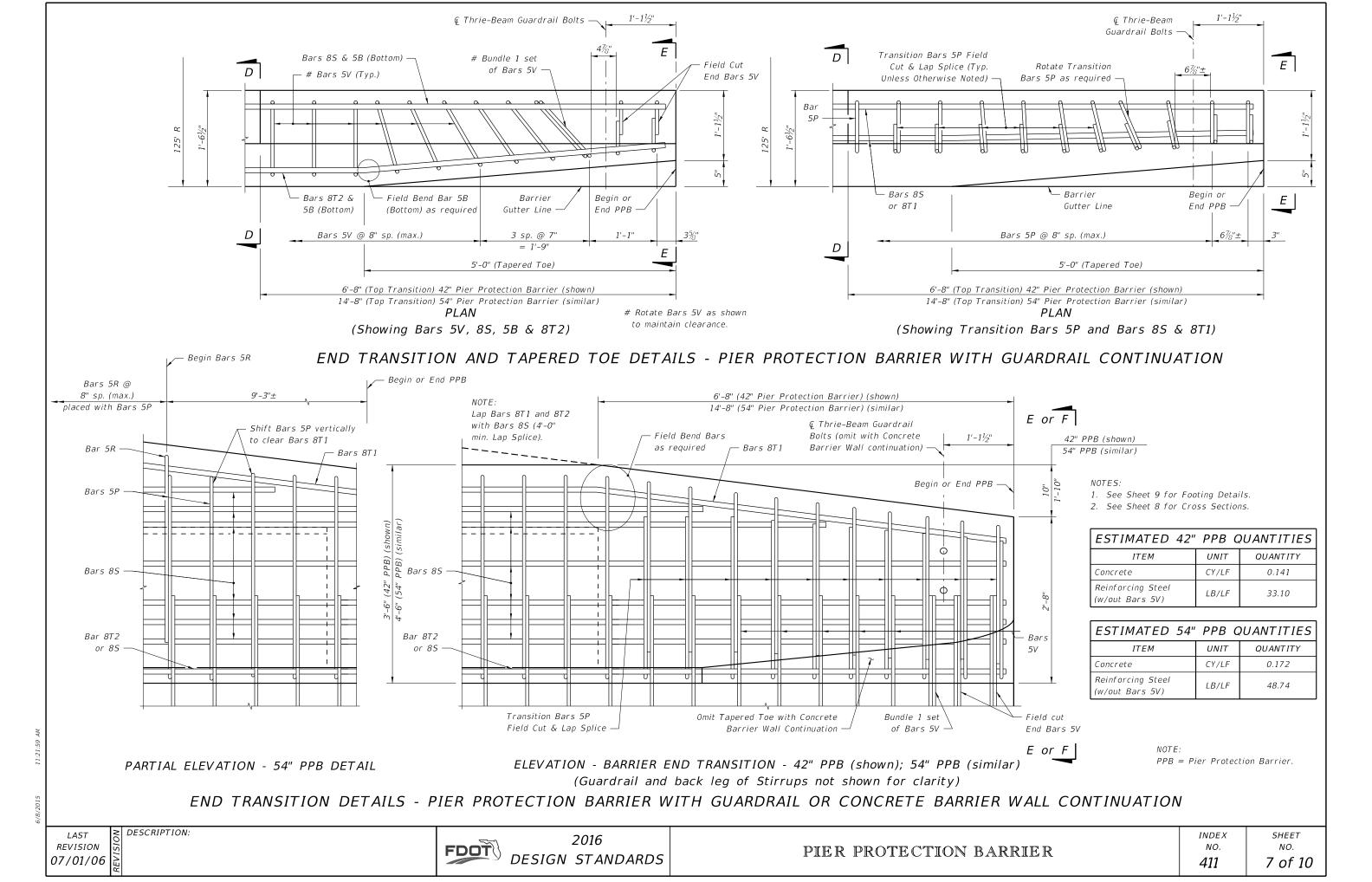
2. Refer to Index No. 400 Detail J for additional guardrail information.

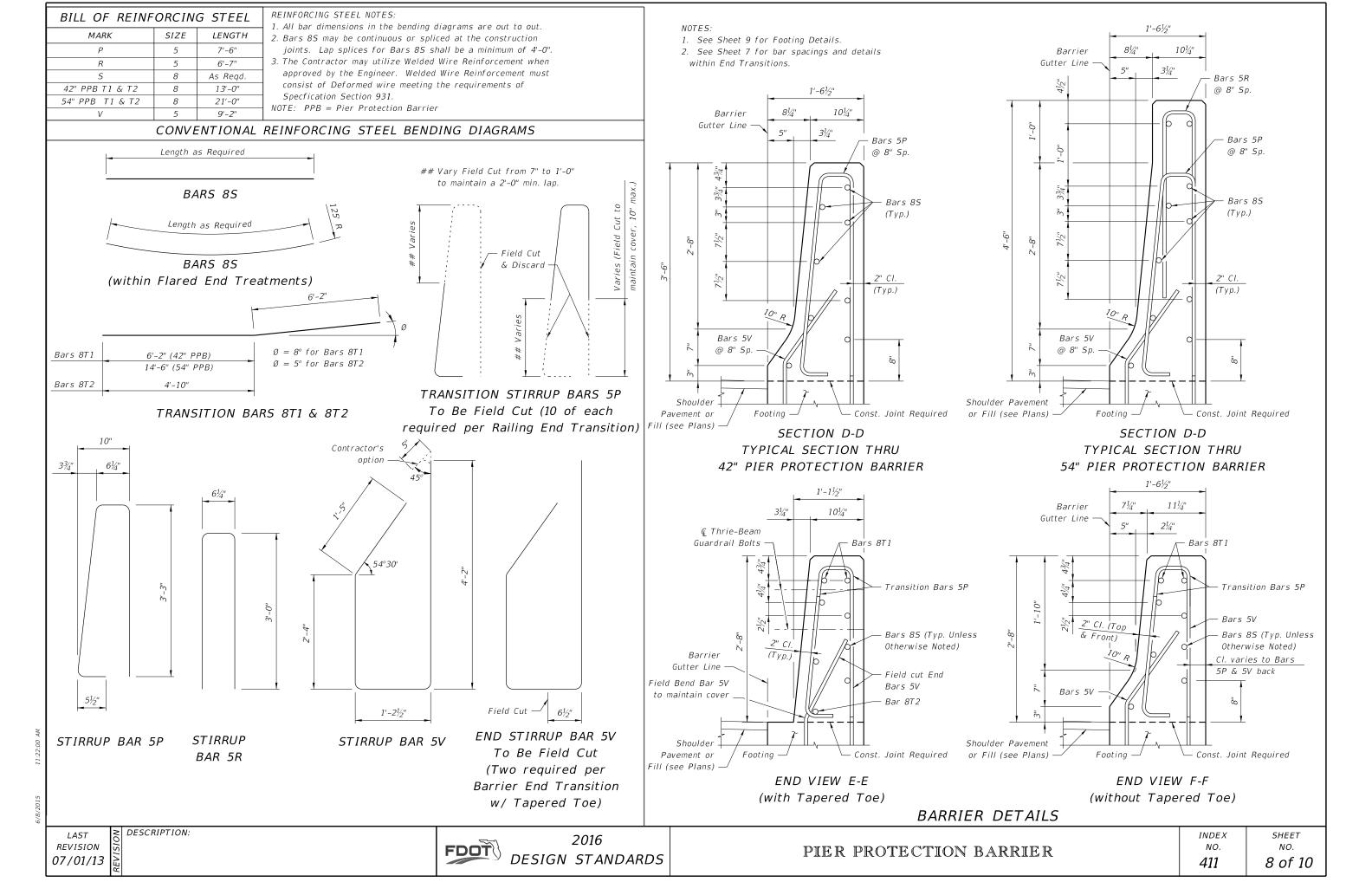
ARC	DISTANCE	OFFSETS "y"	1
ENGTH (Ft.)	"x" (Ft.)	"y" (Ft.)	125
4	4.00	0.06	R
8	7.99	0.26	Y
12	11.98	0.58	X
16	15.96	1.02	Note:
20	19.91	1.60	Barrier may be
21	20.91	1.76	constructed in chords having
24	23.85	2.30	lengths ≤4 feet.
25	24.83	2.49	

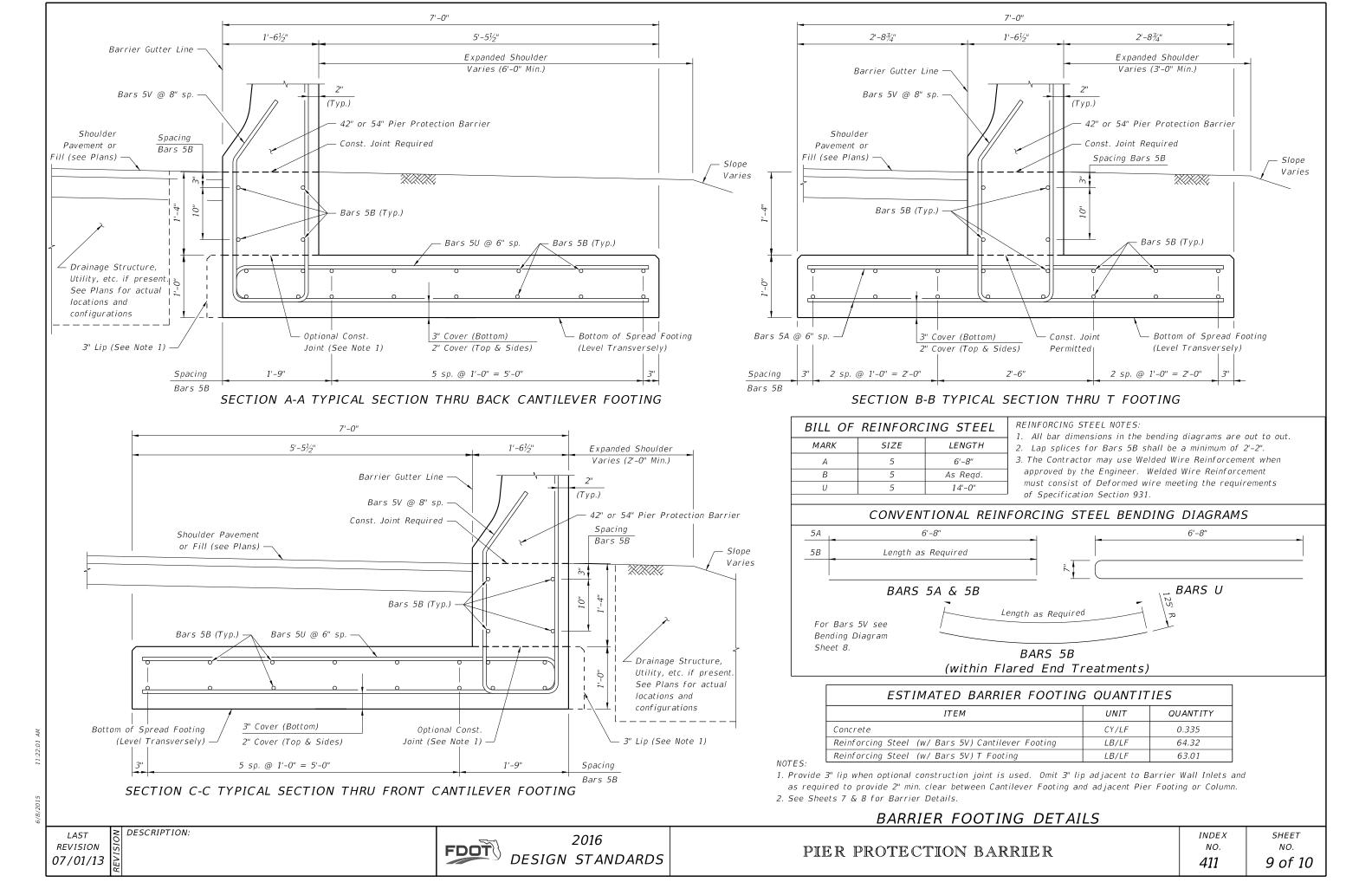
- Extend 32" Concrete Barrier Wall & supporting footing horizontal reinforcing 3' into 42" or 54" Pier Protection Barrier (Typ.) 32" Concrete Barrier Wall, See Index 410

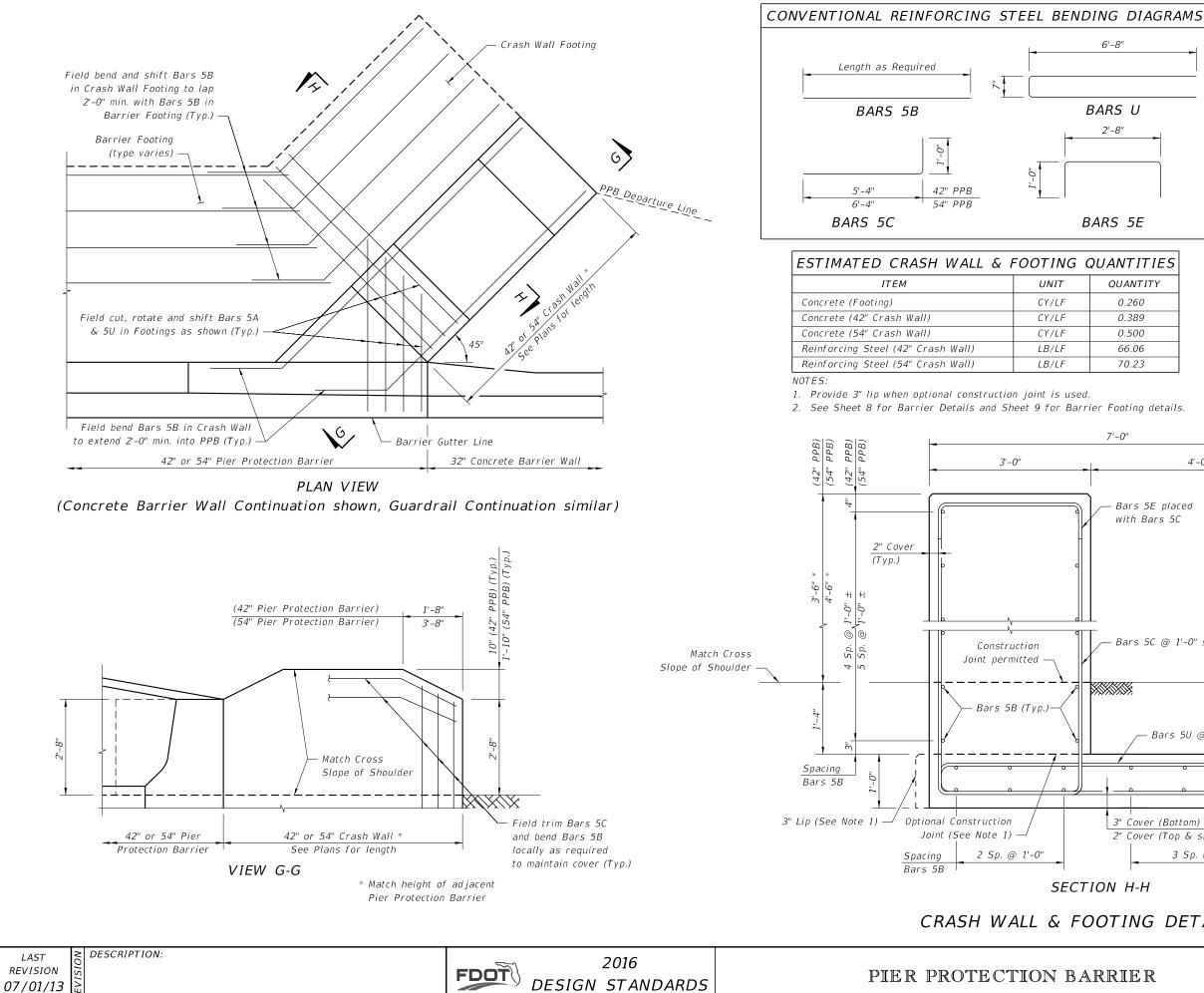
> NOTE: PPB = Pier Protection Barrier.

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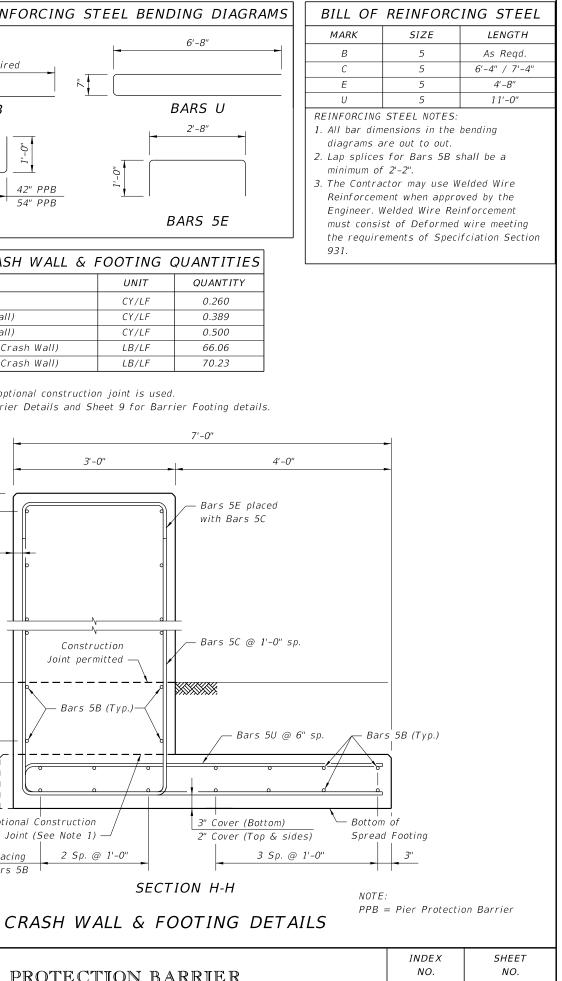








PIER PROTECTION BARRIER



411

10 of 10

6'-8''

2'-8"

7'-0''

UNIT

CY/LF

CY/LF

CY/LF

LB/LF

LB/LF

## GENERAL NOTES

- 1. Pursuant to 35 United States Code, Chapter 18, also known as the Bayh Dole Act of 1980, the non mountable curb was developed through federal funding. The 'Portable Temporary Low Profile Barrier For Roadside Safety' is a licensed design by the University Of Florida. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing (Index No. 412) is provided by the Florida Department Of Transportation solely for use by the Department and its assignees. The purpose for this standard drawing is to indicate the approval of use of the barrier on the State Highway System; to provide sufficient pictorials for identifying the barrier unit; and, to provide general installation geometry for the barrier.
- 3. This legally mandated relationship is unique to federally funded University patents that Department contractors use on Contracts. Pursuant to federal law, the University may pursue royalties for a valid patent. Only those barrier units cast by producers licensed by the University Of Florida will be allowed for installation on the State Highway System in Florida. Barrier wall units shall conform to Section 521 of the Standard Specification and shall be produced in Department-approved plants with quality control plans for precasting concrete barrier walls. Each barrier wall unit shall be permanently marked with an identification that is traceable to the manufacturer, the producing precast concrete plant and the date of production. This permanent identification mark will serve as certification that the unit has been manufactured in accordance with University of Florida drawings and specifications, and the approved quality control program.
- 4. The low profile barrier is to be installed only with hardware and accessories furnished by the licensed barrier producer. Units shall be used for no purpose other than as interconnected segments in a run of barrier. Low profile barrier wall units shall maintain firm contact with adjoining units. Nuts on tensioning rods shall be installed snug tight.
- 5. The low profile barrier is applicable for work zone speeds of 45 mph or less.
- 6. If the plans specify Low Profile Barrier then substitution with other barrier types is not permitted
- 7. Tubular markers shall be orange in color and installed along the run of barrier at the ends and at 50' centers on tangents and 25' centers on radii. The markers shall be fixed to the top of the barrier by an adhesive or other method approved by the engineer. Approach end units shall be marked with a Type I object marker. The cost of the tubular markers and Type I object marker shall be included in the cost of the low profile barrier.
- 8. Information regarding licensing, shop drawings, specifications, quality control and certification of compliance can be obtained from the University Of Florida: Office of Technology Licensing, P.O. Box 115500, Gainesville, Florida, 32611–5500. Telephone: 352–392–8929, Fax: 352–392–6600. Reference UF#11052.
- 9. The Portable Temporary Low Profile Barrier For Roadside Safety shall be paid for under the contract unit price for Barrier Wall (Temporary) Low Profile Concrete, LF, and will be full compensation for furnishing, installing, maintaining and removing barrier wall.
- 10. Deflection space shall be kept clear of any grass, construction debris, stockpiled materials, equipment, and objects.

BACKSIDE AND END PICTORIAL VIEWS

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

LAST	NC
REVISION	SI
07/01/15	2

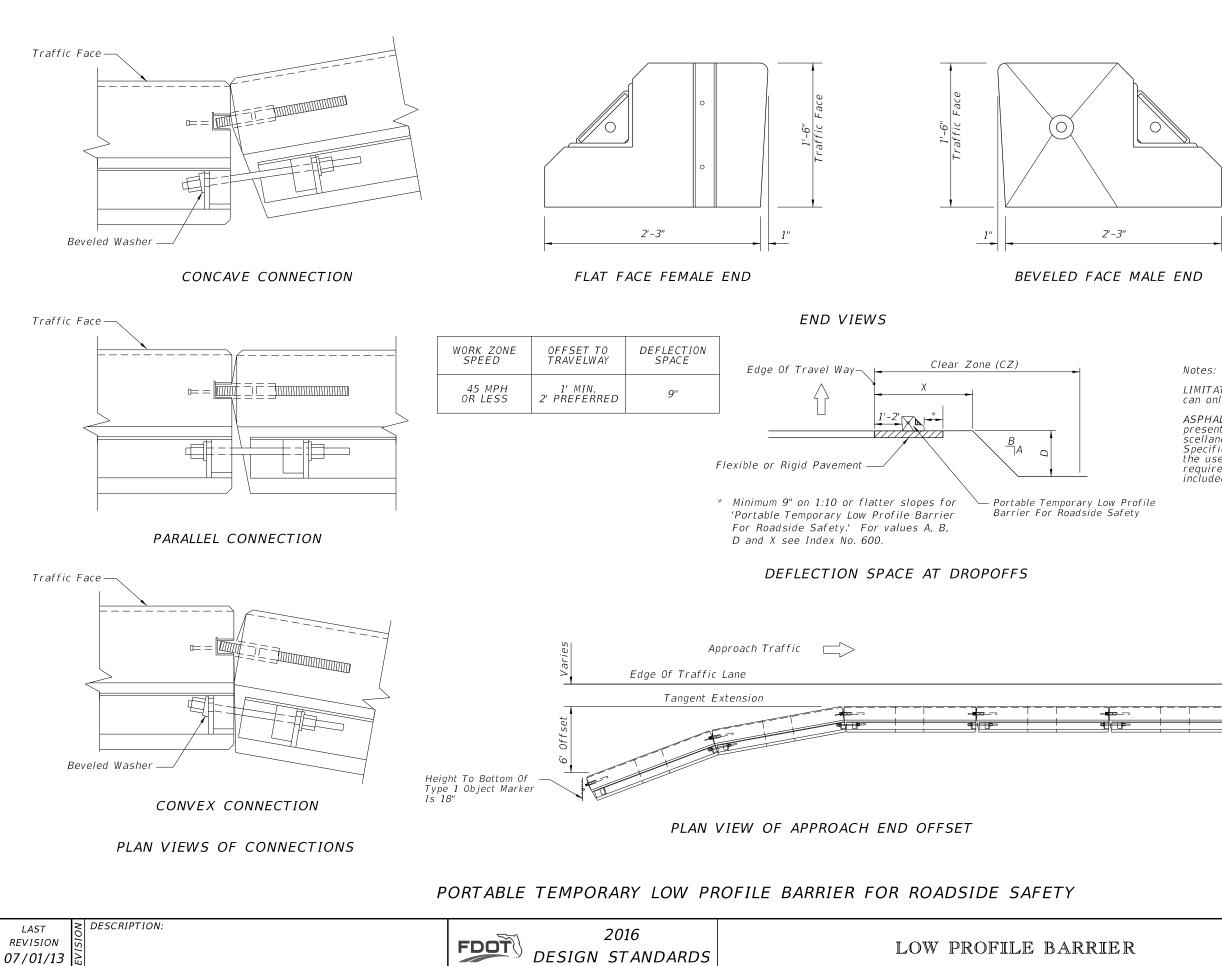
Unit Length 12,00



LOW PROFILE BARRIER

4-81/2

	~
2.7" Slot	
Dran	



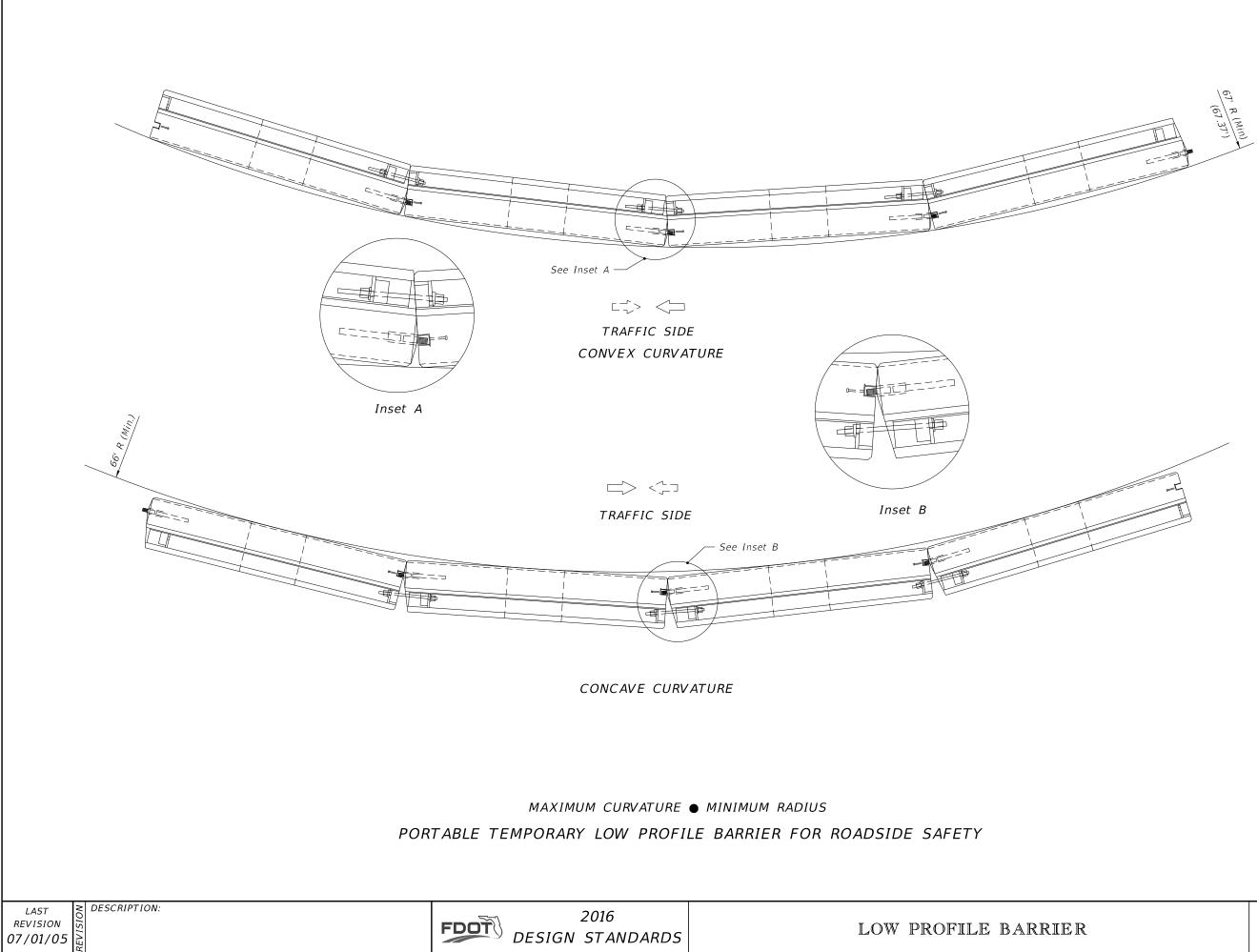
LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement.

ASPHALT PAD: Where exisiting pavement is not present, construct 2" Asphalt Pad using mi scellaneous asphalt pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. Payment for asphalt pad will be included in the cost of the barrier.

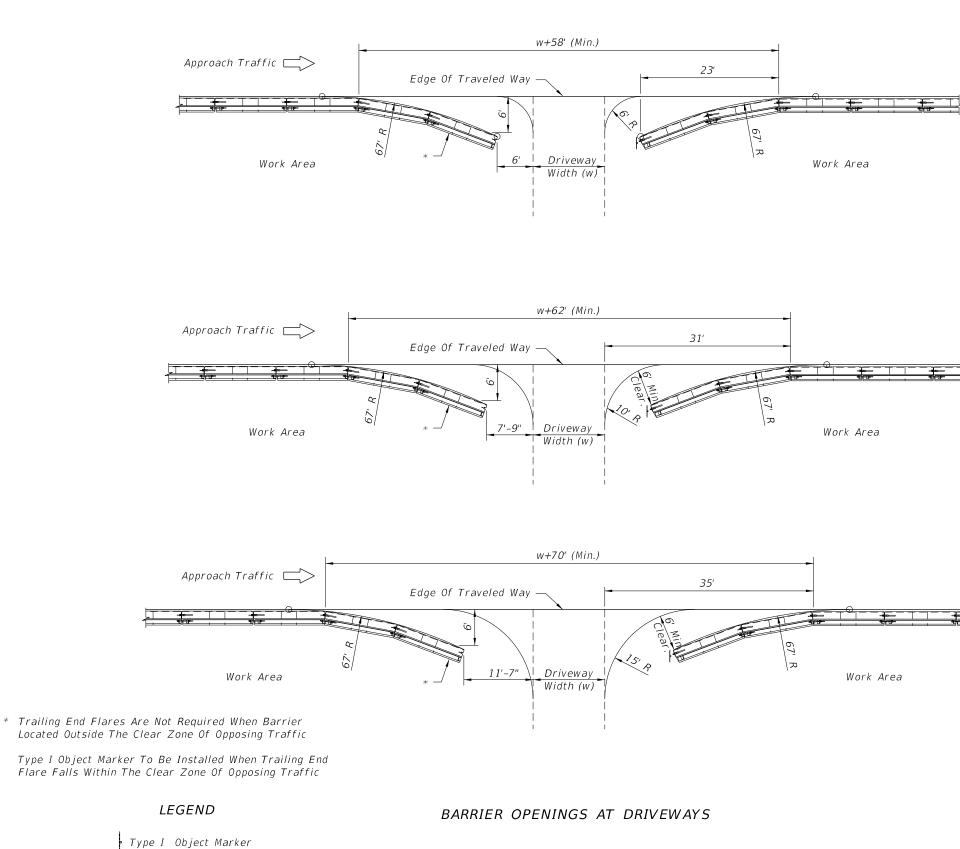
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PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

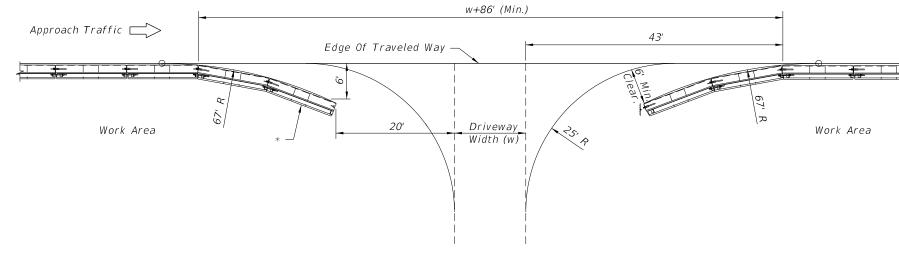
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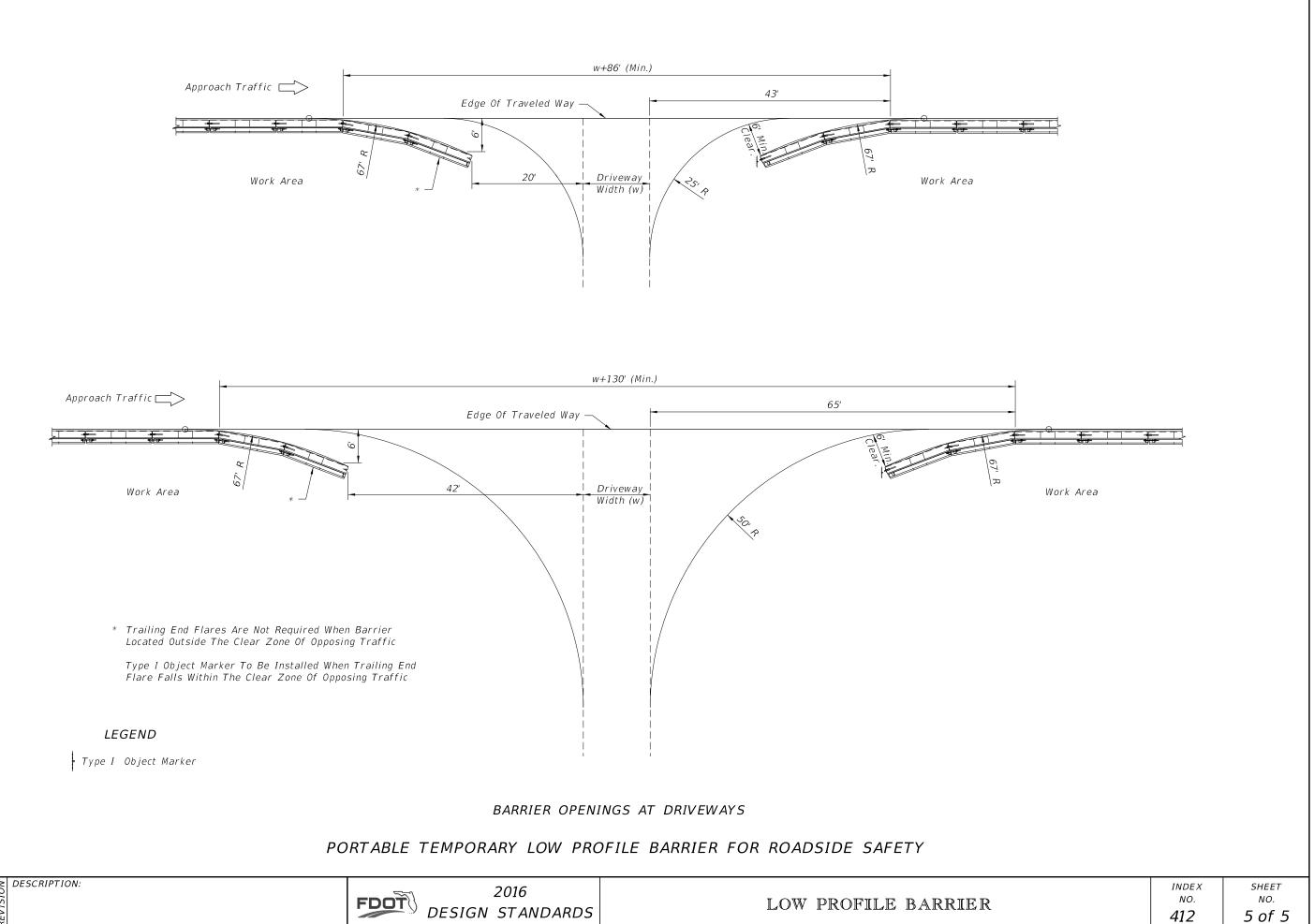
CRIPTION:

2016 FDOT DESIGN STANDARDS

LOW PROFILE BARRIER

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LAST

REVISION

01/01/12

The Type K Temporary Concrete Barrier System has been crash tested to NCHRP Report 350 TL-3 criteria or structurally evaluated to meet the requirements of NCHRP Report 350 TL-3 criteria for the installation configurations as shown utilizing the types, sizes, lengths, shapes, strengths and grades of the fabrication and installation materials as shown.

In order to maintain crashworthiness of the system, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

### FABRICATION NOTES:

- FABRICATOR PREQUALIFICATION: The Barrier Units shall be made in a prestressed concrete plant that meets the requirements of Specification Section 450 or in a precast plant meeting the requirements of Specification Section 105.
- CONCRETE: Concrete shall be Class IV in accordance with Specification Section 346. Specification Sections 346-10.2 through 346-10.4 are not applicable. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.
- REINFORCING STEEL: All reinforcing steel shall be ASTM A 615, Grade 60 except for Bars 6D1, 6D2 and 6D3. Bars 6D1, 6D2 and 6D3 shall be ASTM A 706 except that a  $2\frac{3}{4}$ " diameter pin must be used for the 180 degree bend test. After fabrication, all or part of Bars 6D shall be hot dip galvanized in accordance with Specification Section 962 or coated with a cold galvanizing compound in accordance with Specification Section 975. The minimum limit of galvanizing or coating is shown in the Bending Diagrams. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated. Install Bars 6D within  $\frac{1}{6}$ " of the plan dimensions. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.

At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with Specification Section 931 and the details shown on Sheet 2 may be utilized in lieu of Bars 4A and 5B.

All dimensions in the Bending Diagrams are out to out. All reinforcing steel shall have a 2" minimum cover except as noted.

- LIFTING SLEEVE ASSEMBLY: Inclusion of the Lifting Sleeve Assemblies is optional. Steel for Pipe Sleeve shall be in accordance with ASTM A 53. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.
- SURFACE FINISH: Construct Barrier Units in accordance with Specification Sections 400 and 521. Finish the top and sides of the Barrier Units with a General Surface Finish. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish. Use stationary metal forms or stationary timber forms with a form liner.

MARKING: Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall. Ink stamps are not allowed. Permanently mark with the following information:

- Туре К1
- Fabricator's name or symbol
- Date of manufacture (day, month and year)

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

Alternate Designs: Manufacturers/vendors seeking approval of proprietary Temporary Barrier Systems for inclusion on the Approved Products List (APL) as alternative designs shall submit a Product Application package. The application package shall include manufacturer's product drawings, specifications, installation manual, National Cooperative Highway Research Program (NCHRP) Report 350 or Manual for Assessing Safety Hardware (MASH) Test Level 3 (TL-3) crash test documentation and the FHWA "Letter of Acceptance." The posted APL drawings will need to include the following:

- 1. Anchorage, bolting, and staking details for connections to asphalt and concrete pavement.
- 2. Sections and tables showing required deflection space and minimum offsets to above ground hazards or drop-offs.
- 3. Alignment and length of need details.
- 4. Transition and overlap details.

FDOT

5. End treatment details.

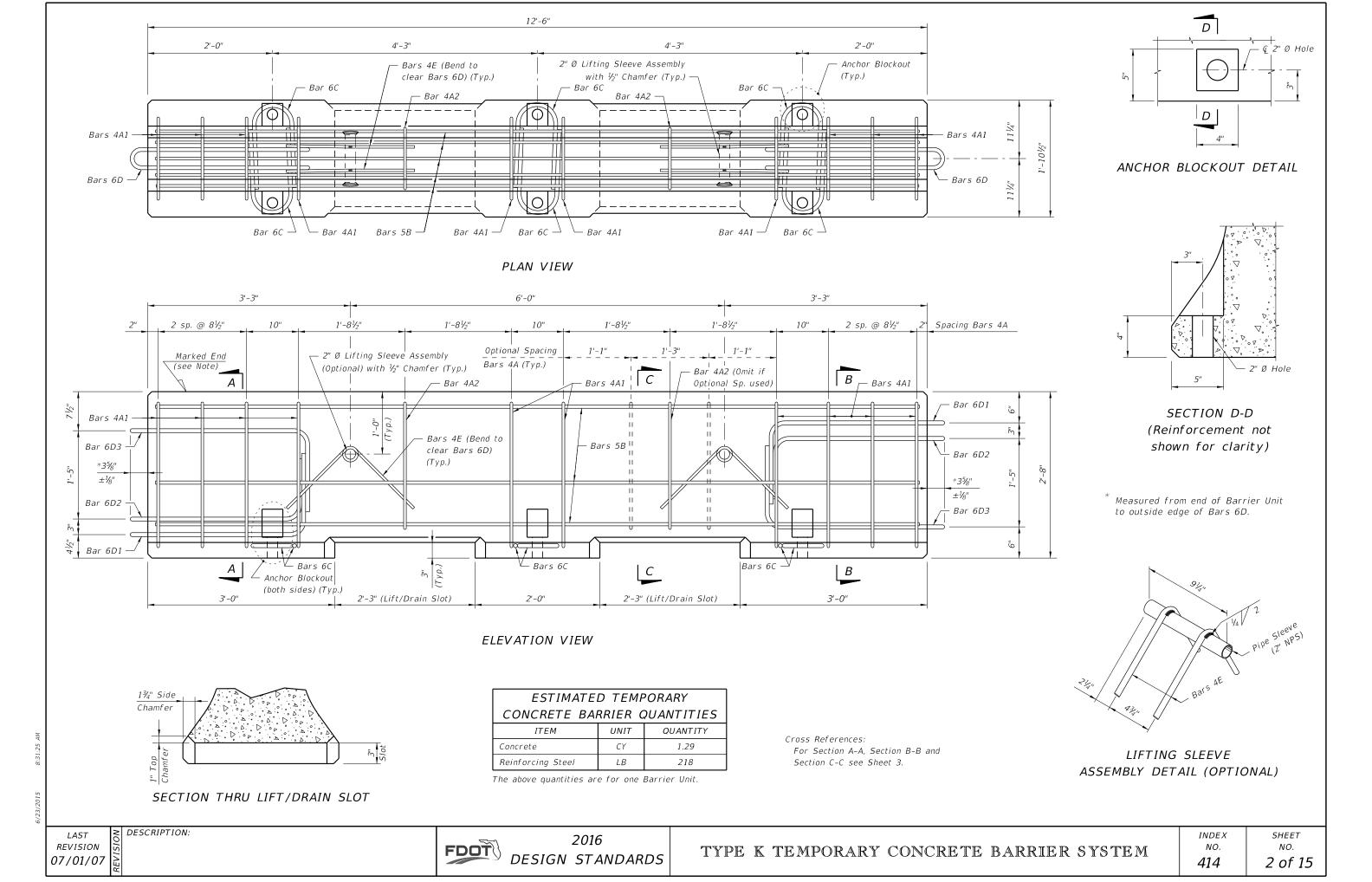
5 8:31:21 A

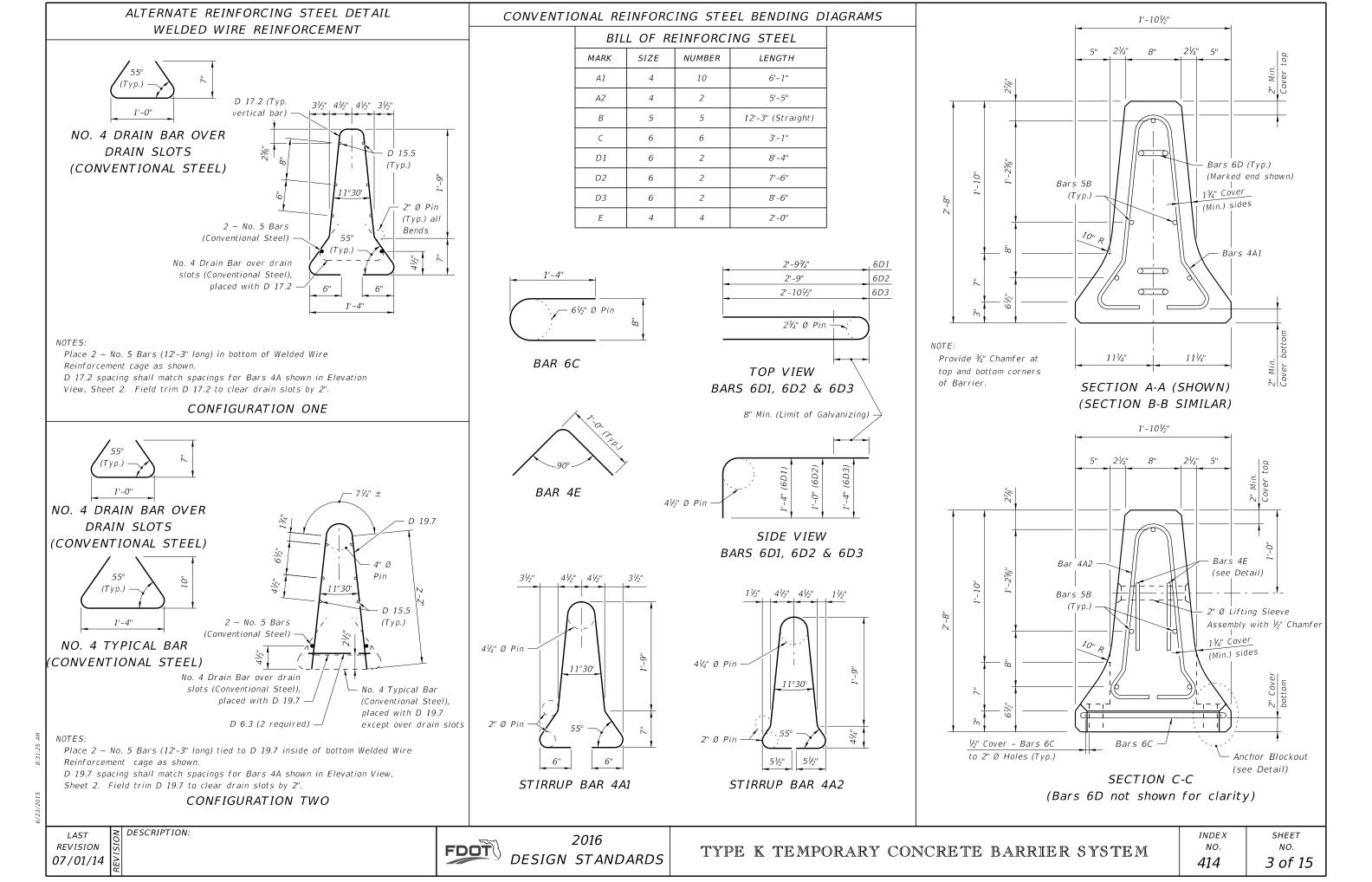
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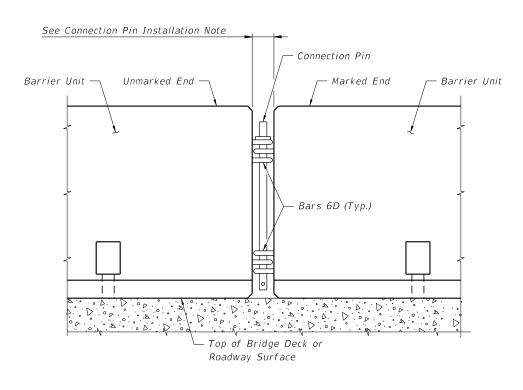
2016 DESIGN STANDARDS TYPE K

TYPE K TEMPORARY CONCRETE BARR

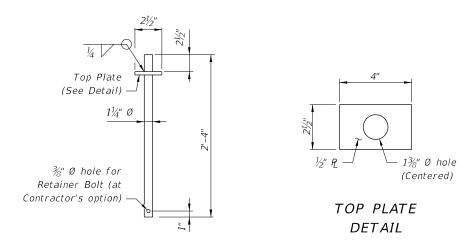
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DETAIL OF CONNECTION BETWEEN BARRIER UNITS



CONNECTION PIN DETAIL

### NOTES FOR ALL INSTALLATIONS:

- 1. LIMITATION OF USE: This Temporary Concrete Barrier System is intended for work zone traffic control and other temporary applications. It shall not be used for permanent traffic railing construction unless specifically permitted by the Plans. Except as shown for the Back Filled Roadway Installations, the Barrier Units must be installed on a flexible pavement (asphalt) or rigid pavement (concrete) surface as shown with a cross slope of 1:10 or flatter. Except as shown for transition installations, Type K Barrier Units are not intended to be bolted down or staked down in locations where they can be impacted from the back side.
- 2. HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.
- 3. SURFACE PREPARATION: Except as shown for the Back Filled Roadway Installations, remove all grass, debris, loose dirt and sand from the pavement, bridge deck or Asphalt Pad surface within the barrier footprint just prior to placement of the Barrier Units.
- 4. OFFSET TO TRAVELWAY: Offset shall meet requirements as shown on sheet 1 of Index 415.
- 5. CONNECTION PIN ASSEMBLY: Steel for Connection Pin and Top Plate assemblies shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Nondestructive testing of welds shall not be required. At the Contractor's option, a 🔏 diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.
- 6. CONNECTION PIN INSTALLATION: Initially set Barrier Units by using a  $3\frac{5}{8}$ " wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 5). Barrier Units shall not be used unconnected.
- 7. DELINEATION: Mount Barrier Delineators on top of Barrier Units that are used as traffic barriers along travel ways in work zones. Space the Barrier Delineators at 50' centers in alignment transitions, 100' centers on horizontal curves and 200' centers on tangent alignments.
- 8. MAINTENANCE: Deflection space shall be kept clear of any grass, construction debris, stockpiled materials, equipment, and objects.
- 9. REUSE OF CONNECTION PINS: Connection pins may be reused if they have the structural integrity of new pins.
- 10. INSTALLATIONS ON CURVED ALIGNMENTS: The details presented in these Standards are shown for installations on tangent alignments. Details for horizontally curved alignments are similar.
- 11. TRANSITIONS: Transitions are required between freestanding, bolted down, staked down and back filled Type K Barrier installations, see Sheet 8 for transition requirements and details. Transitions are also required between installations of Type K Barrier and other types of temporary barrier, see Index No. 415 for transition requirements and details. Splices and transitions are required between installations of Type K Barrier and permanent Bridge or Roadway Traffic Railings, see Sheets 9 through 13 for transition requirements and details. Transitions are required between installations of Type K Barrier and Proprietary (APL) Barrier Systems, See Sheets 14 and 15 for transition requirements and details.
- 12. PAYMENT: Barrier Units for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary) (F&I) (Type K), LF. Any relocation of the Barrier Units required for the project shall be paid for under the contract unit price for Barrier Wall (Temporary) (Relocate) (Type K), LF. The Contractor shall furnish Barrier Units except when the Plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return. Unless otherwise noted on the Plans, the Barrier Units shall become the property of the Contractor and shall be removed from the site prior to acceptance of the completed project.

### NOTES FOR THRIE BEAM GUARDRAIL SPLICE INSTALLATIONS:

- 1. THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices in accordance with AASHTO M 180, Type II (Zinc coated) and as follows: Two panels per splice (One panel per side) of Class B (10 Gauge), or
  - Four panels per splice (Two nested panels per side) of Class A (12 Gauge).

Guardrail panel length shall be 12'-6". Provide and install all other associated metallic guardrail components (Terminal Connectors, Shoulder Bolts, Hex Bolts and Nuts. Filler Plates. etc.) in accordance with Index No. 400.

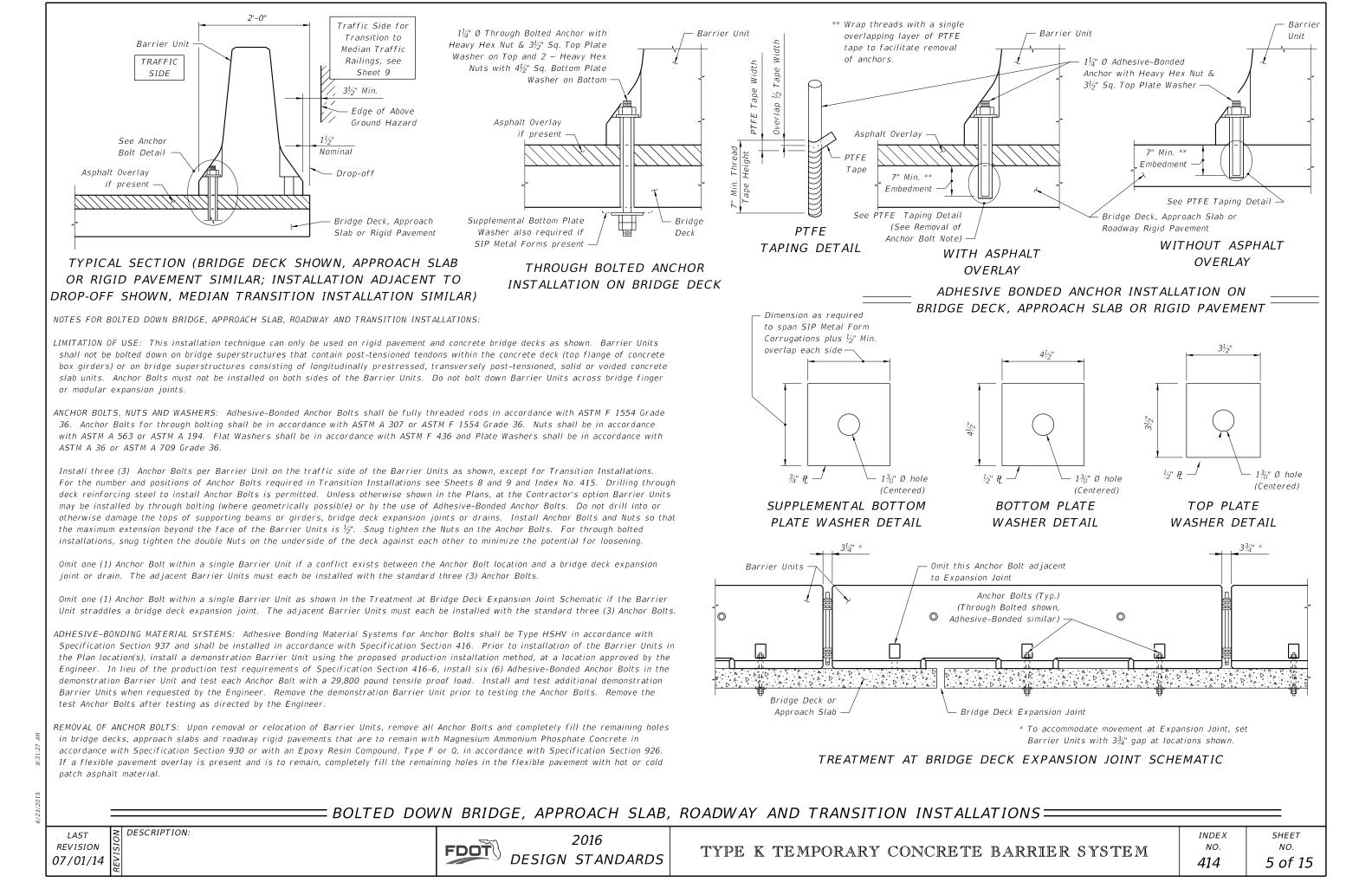
Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted. Do not drill or cut through utilities or conduits within permanent concrete traffic railings.

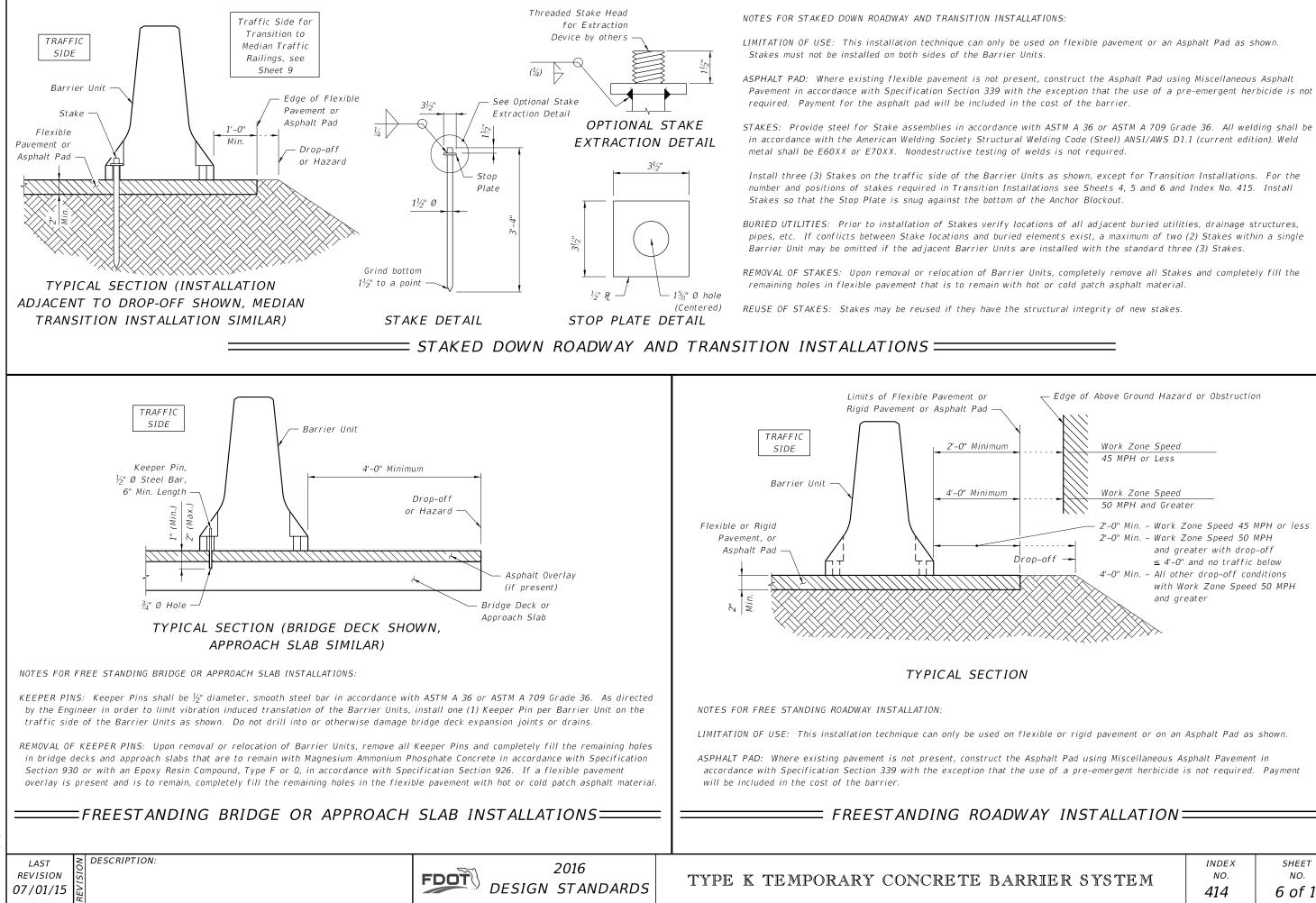
- 2. GUARDRAIL OFFSET BLOCKS: Provide and install timber Offset Blocks meeting the material requirements of Index No. 400. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.
- 3. CONCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification Section 346, any Class, or a commercially available prebagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification Section 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.

DESCRIPTION:

FDOT

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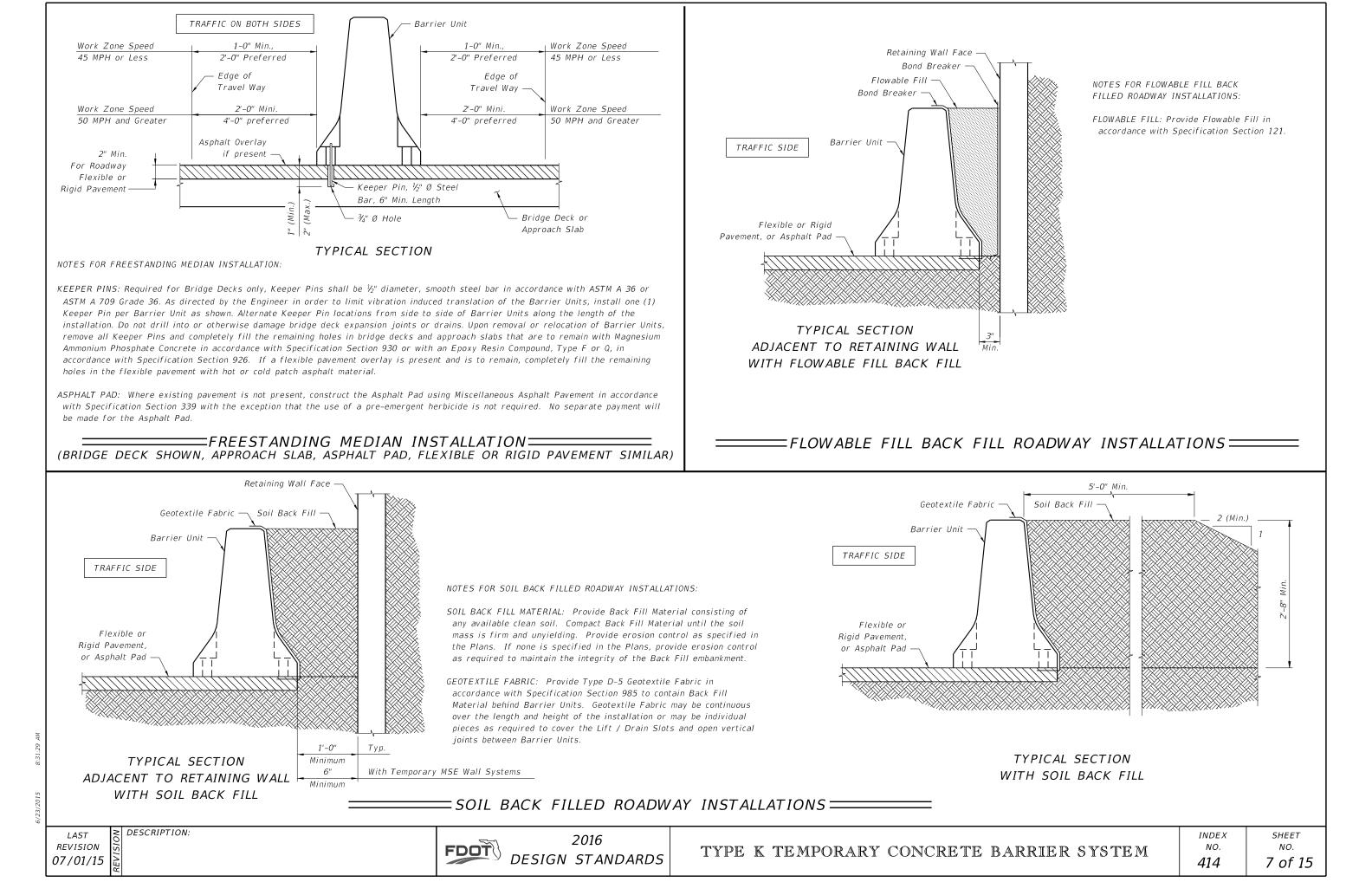


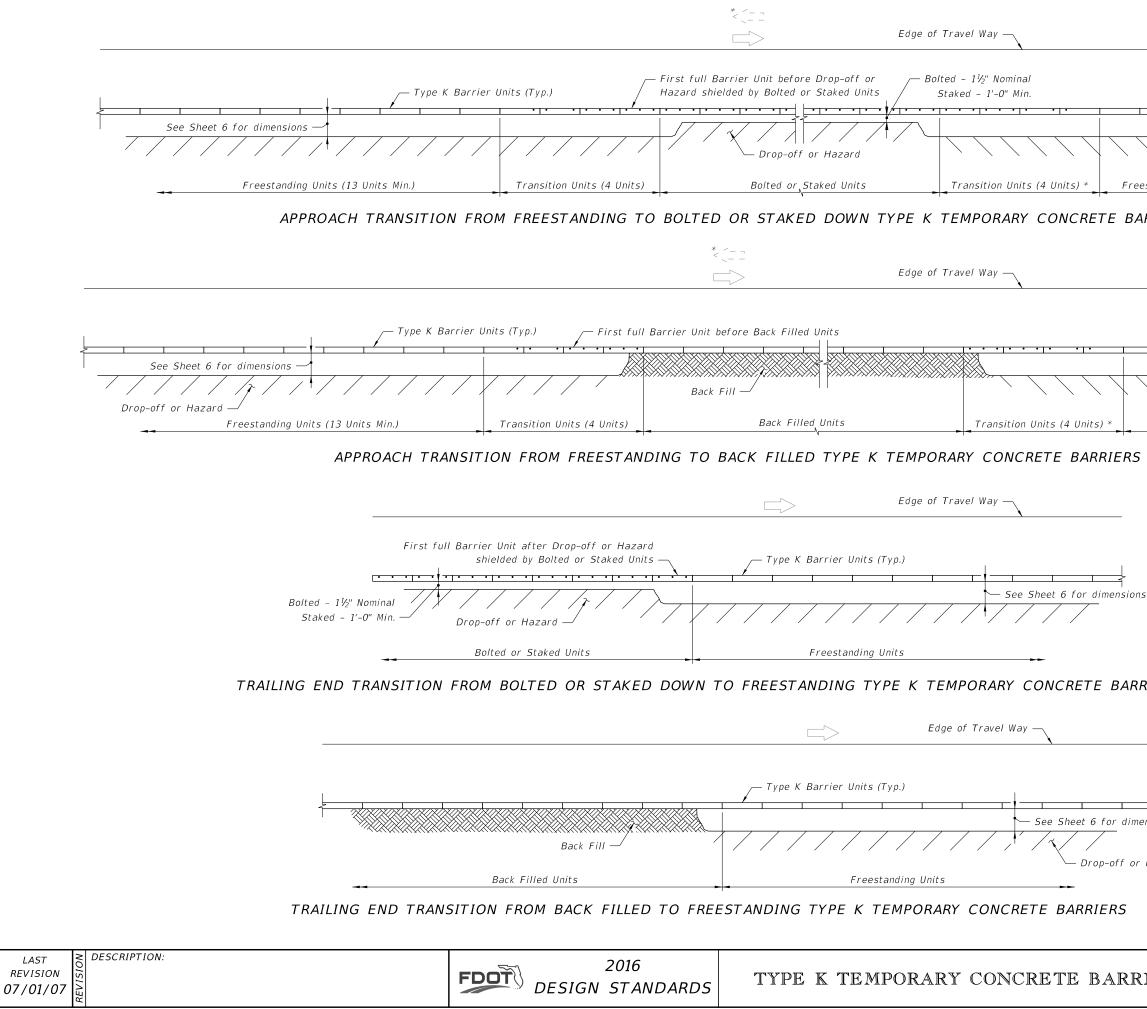


$\sum$ Edge of	Above Ground Hazard or Obstruction
-1///.	Work Zone Speed
	45 MPH or Less
	Work Zone Speed
	50 MPH and Greater
	- 2'-0" Min Work Zone Speed 45 MPH or less
-off -	2'-0" Min Work Zone Speed 50 MPH and greater with drop-off ≤ 4'-0" and no traffic below
	4'-0" Min All other drop-off conditions with Work Zone Speed 50 MPH and greater

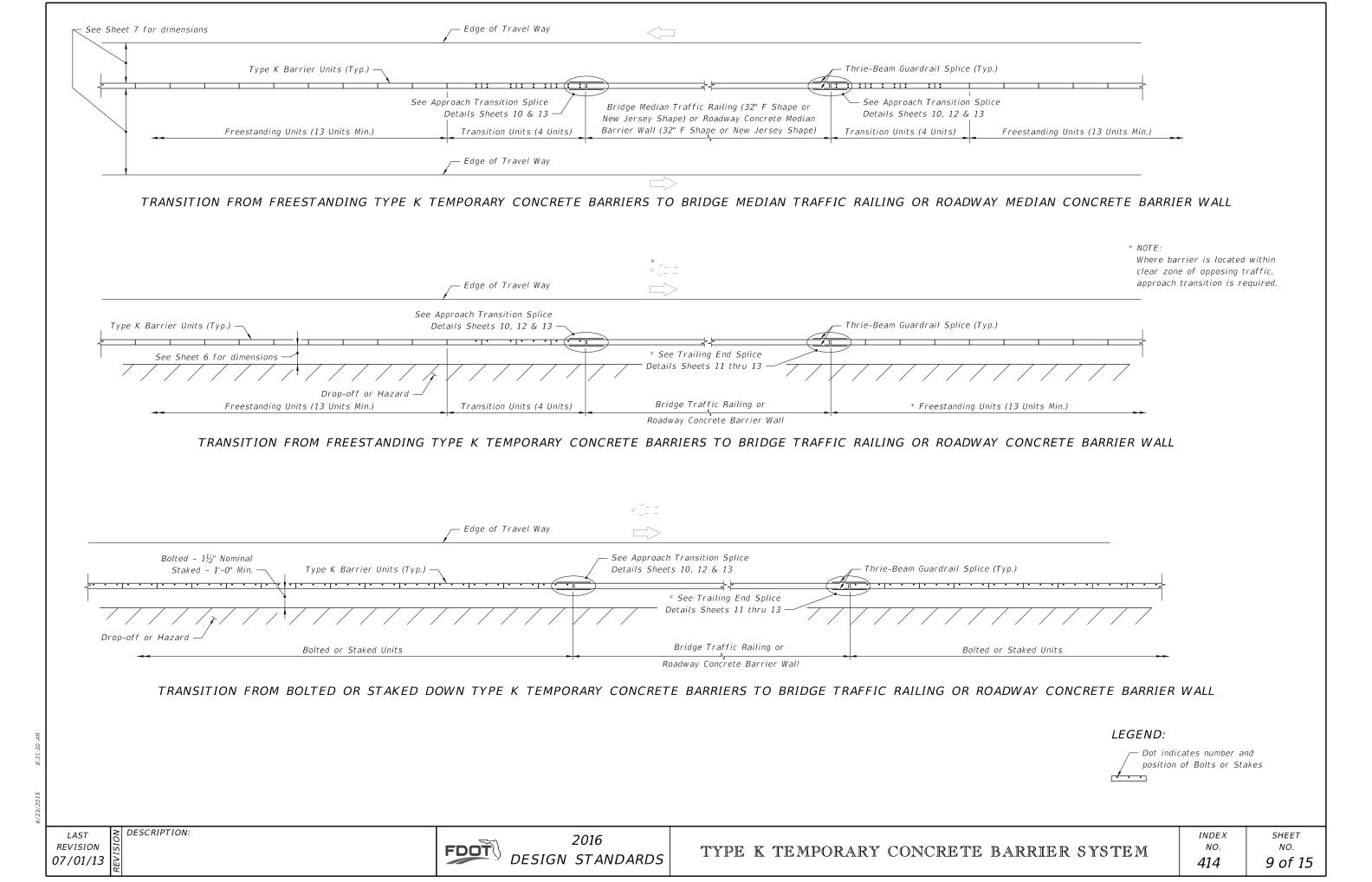
Y	INSTALLATION
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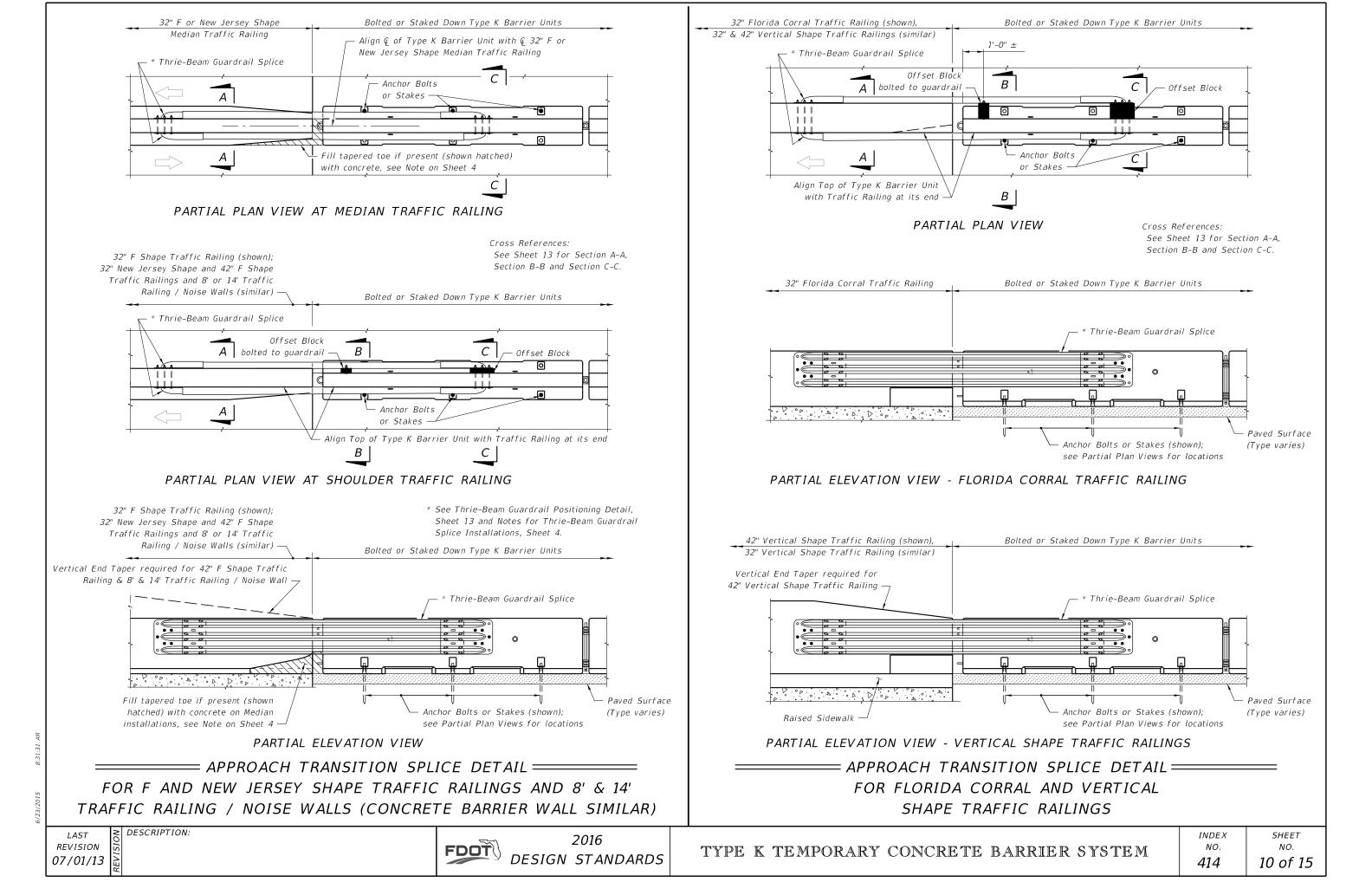
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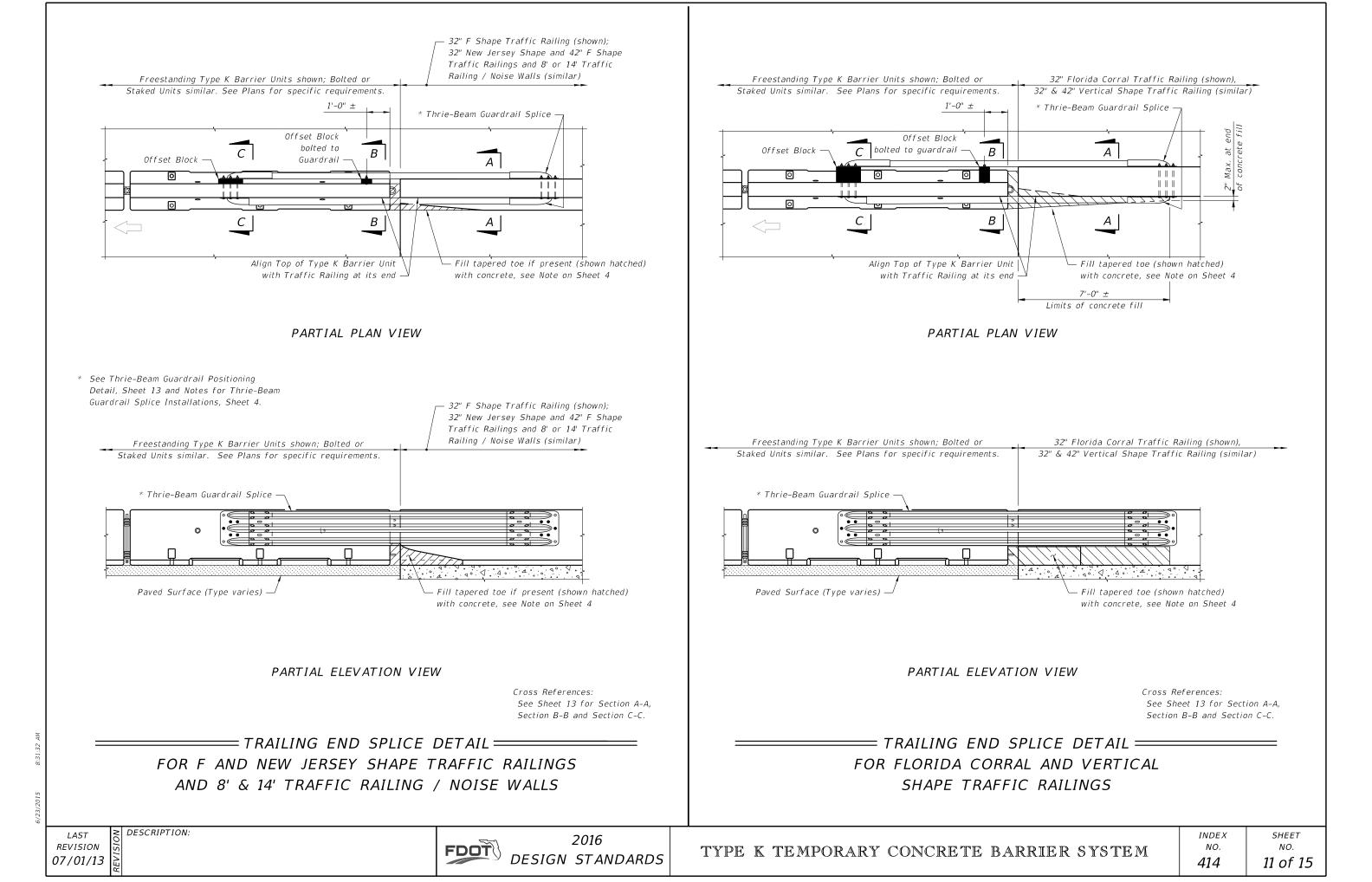


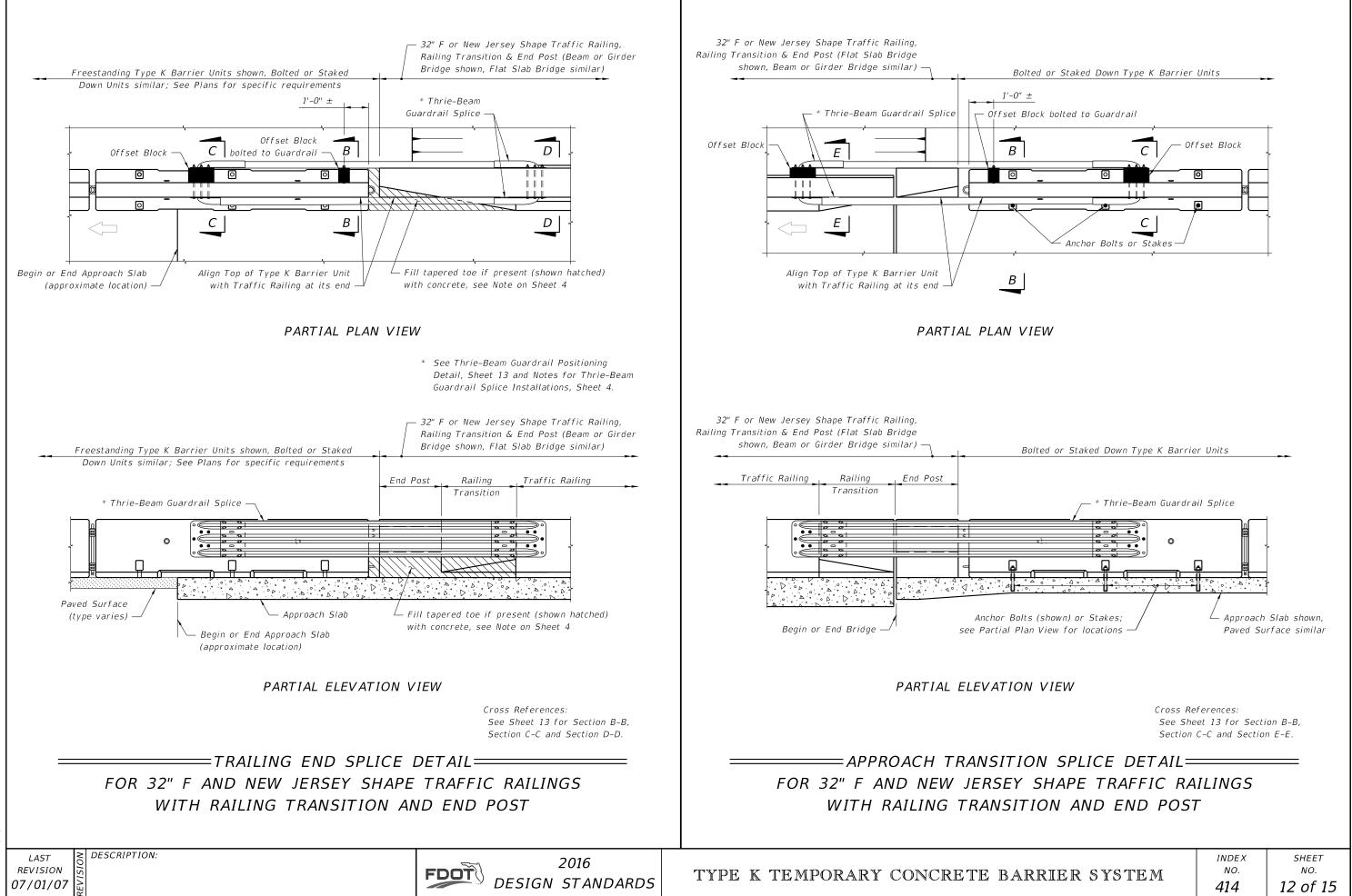


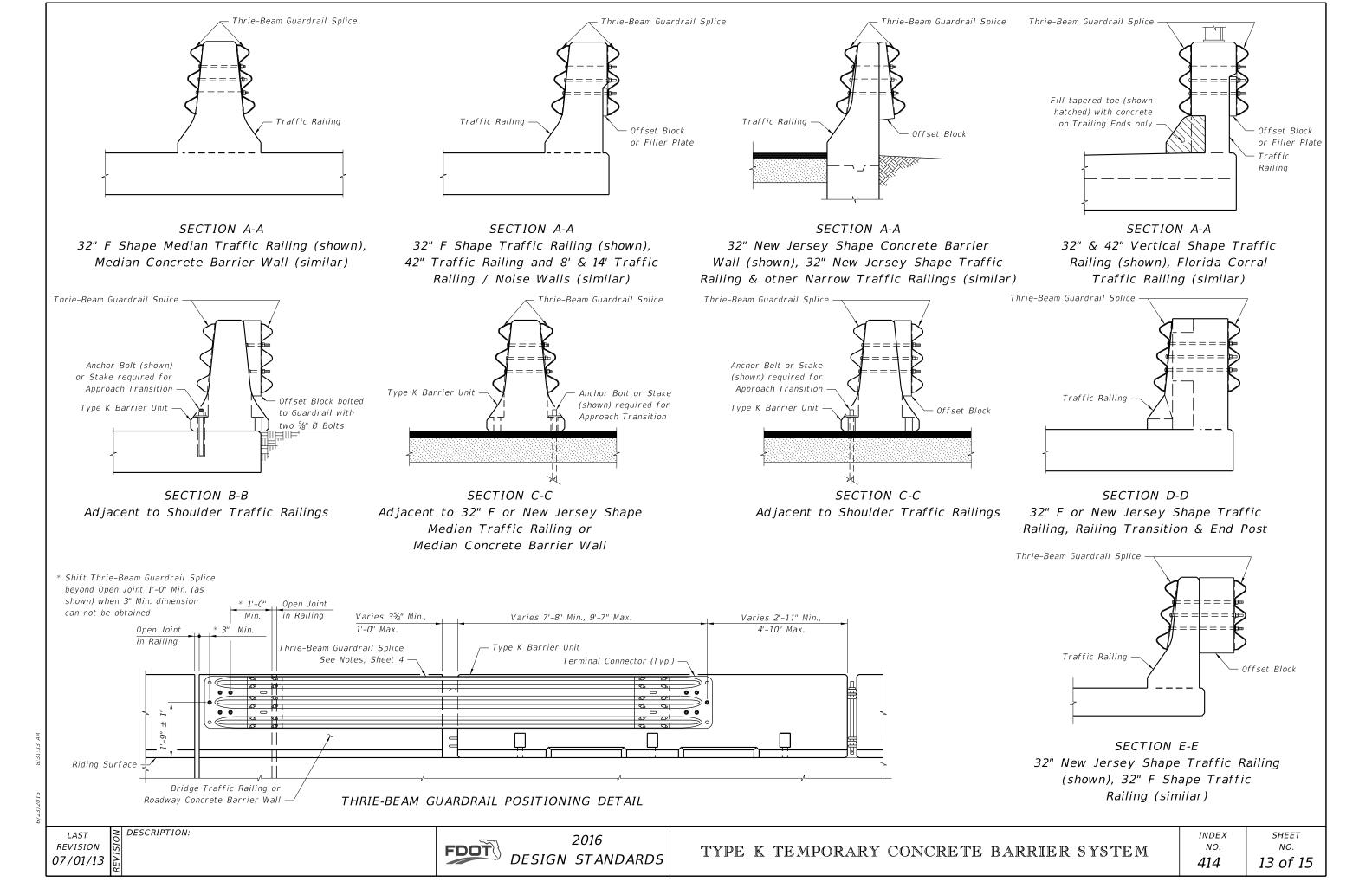
estanding Units (13 Units Min.) *		
RRIERS		
* NULE: Where Ba Clear Zon	nrrier is located ne of opposing Transition is r 	traffic,
	_	
	`	
Freestanding Units (13 Units M	in.) *	
LEGEI	ND:	
/— l	Dot indicates nu position of Bolt.	
		s of Stakes
5		
RIERS		
_		
 ensions		
Hazard		
IER SYSTEM	index NO. <b>414</b>	<sup>sheet</sup> NO. <b>8 of 15</b>

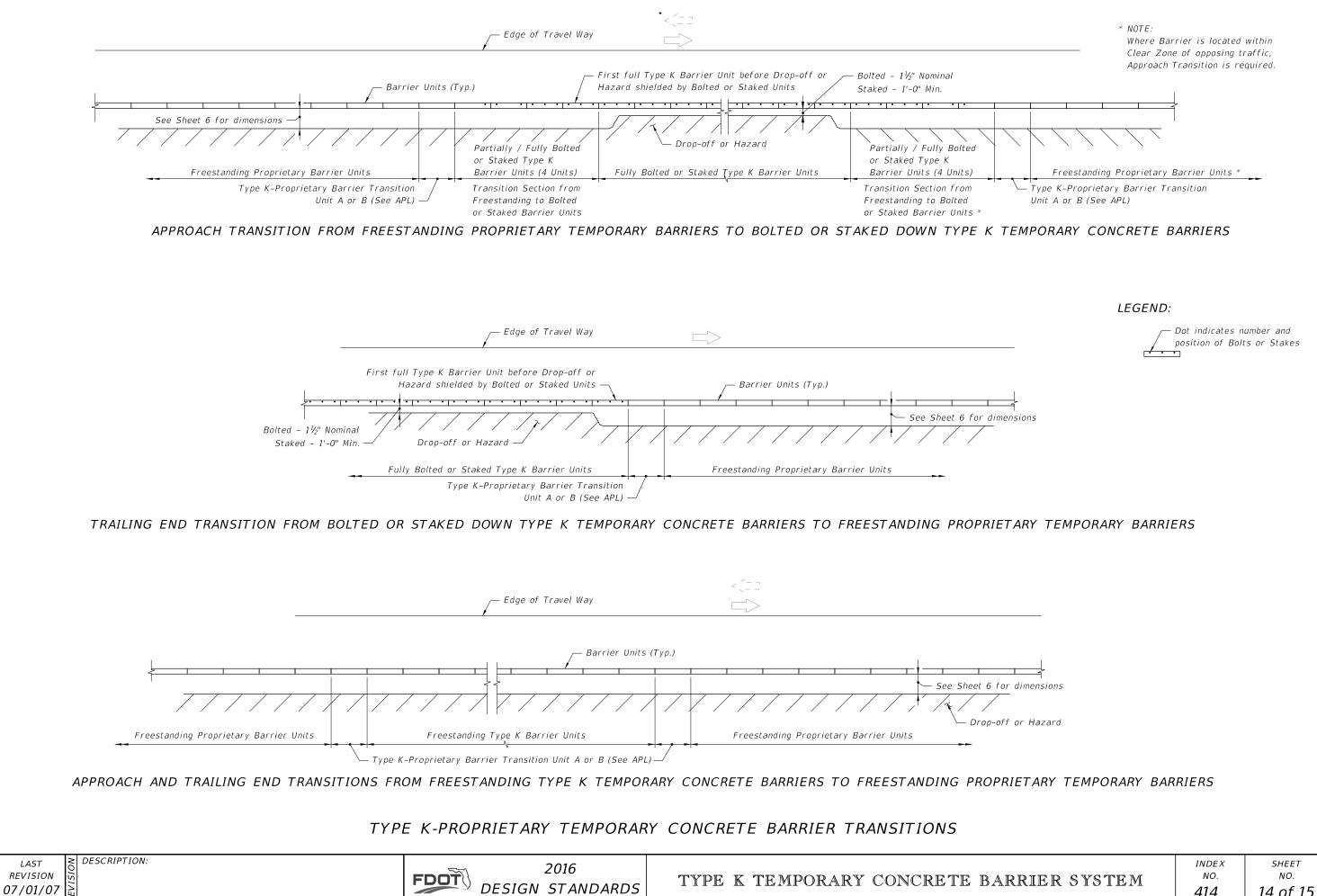




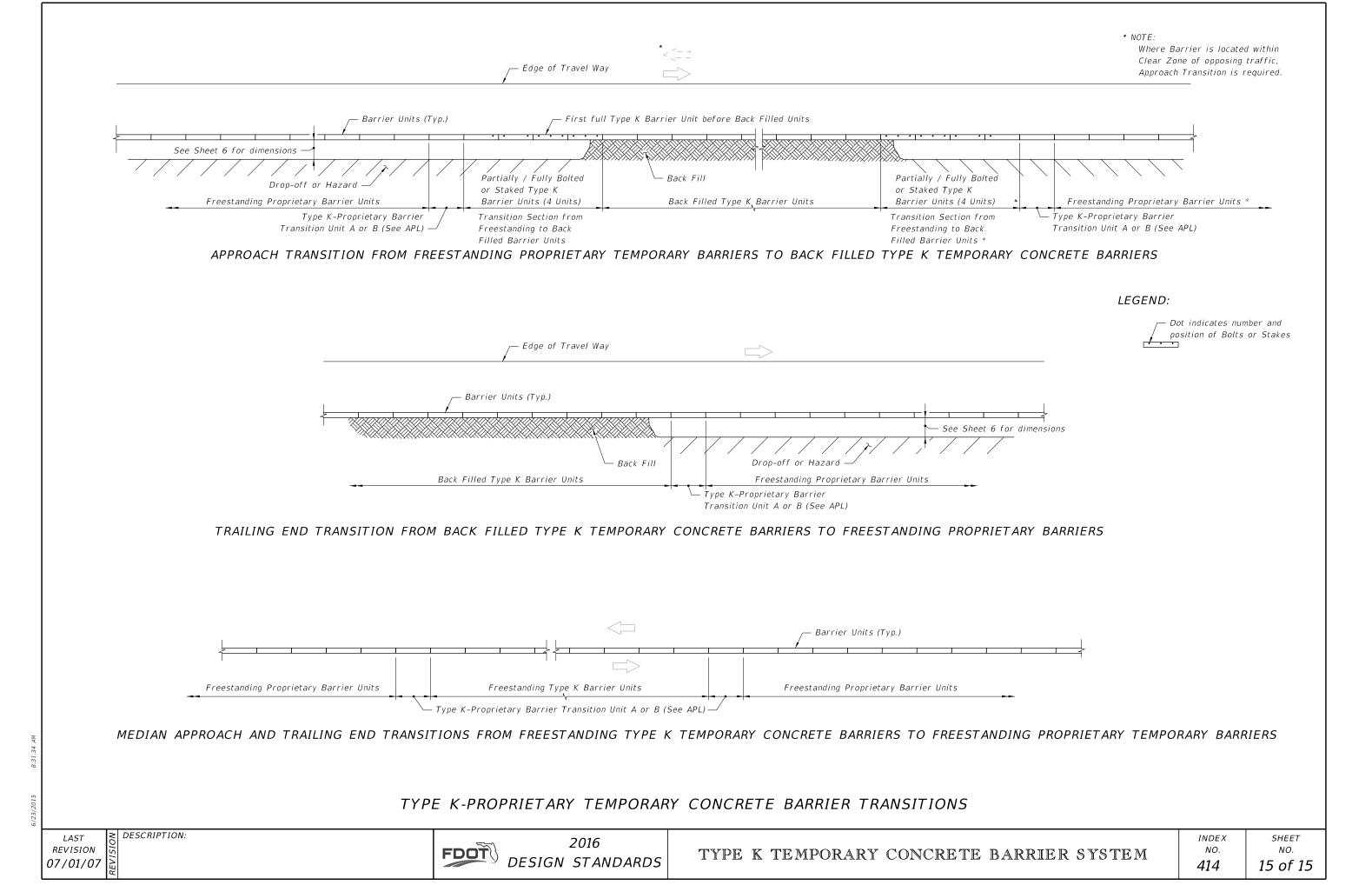








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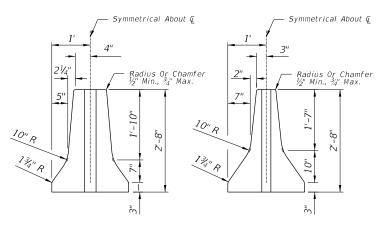


# GENERAL NOTES

- 1. Temporary concrete barrier systems on roadways may be any of the following:
- a. The FDOT Type K Temporary Concrete Barrier system (Design Standard Index 414). F-Shape Units. For temporary concrete barrier systems on bridges see Design Standard Index No. 414.
- b. Proprietary temporary concrete barrier systems meeting NCHRP Report 350 Test Level 3 criteria which are included on the Approved Products List.
- 2. Barrier units of dissimilar types may be interconnected within a single line barriers using transition units.
- 3. Alignment, length of need, anchorage and end treatment shall be in accordance with this Index.
- 4. Temporary concrete barrier units shown herein shall not be used for permanent barrier wall construction regardless of unit length.
- 5. If the plans specify Barrier (Temporary) (Type K), substitution with other barrier types is not permitted.
- 6. If the plans specify temporary concrete barrier system, substitution with water filled barriers is not permitted.
- 7. Where existing pavement is not present, construct an Asphalt Pad using Miscellaneous Asphalt Pavement. Cost of the Asphalt Pad to be included in the cost of the Barrier system.
- 8. Barrier Delineators meeting the requirements of Specifications Section 993 are to be mounted on top of temporary concrete barriers that are used as barriers along traveled ways in work zones. The barrier delineators are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways. Color must match adjacent longitudinal pavement marking.
- 9. Barrier units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier (Temporary), LF.
- 10. Deflection space shall be clear of any grass, construction debris, stockpiled materials, equipment, and objects.
- 11. Placing alternate temporary barrier systems with heights greater than 32 inches within the work zone may obstruct the clear sight distance at intersections and driveways. Prior to placing these barrier systems, the contractor shall submit a Certification Statement that the clear sight distance meets the requirements of Index 546, signed and sealed by a Florida Professional Engineer.

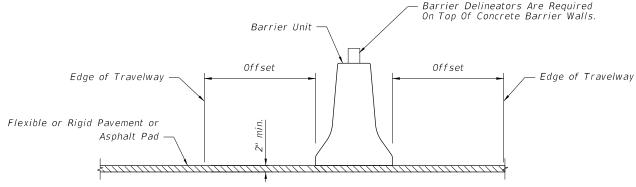
12. Minimum temporary concrete barriers installed per run shall be 16 units.

	OFFSET AND DEFLECTION SPACE REQUIREMENTS			
Installation Shielding Work Zone Sheed				Deflection Space
Above 45 mph or Less Ground		1' min, 2' preferred	2' min.	
Left or Right Shoulder Drop-Off Hazards	Hazards	50 mph and Greater	2' min, 4' preferred	4' min.
		45 mph or Less	1' min, 2' preferred	2' min.
	Drop-Off	50 mph and Greater		
	Hazards	a. Drop-offs 4' or Less and NO traffic below	2' min, 4' preferred	2' min.
	b. All drop-off conditions other than 'a'	2' min, 4' preferred	4' min.	
Separating Adjacent Opposing	45 mph or Less	1' min, 2' preferred	1' min., 2' prefered	
Traffic Traffic		50 mph and Greater	2' min, 4' preferred	2' min., 4' preferrea

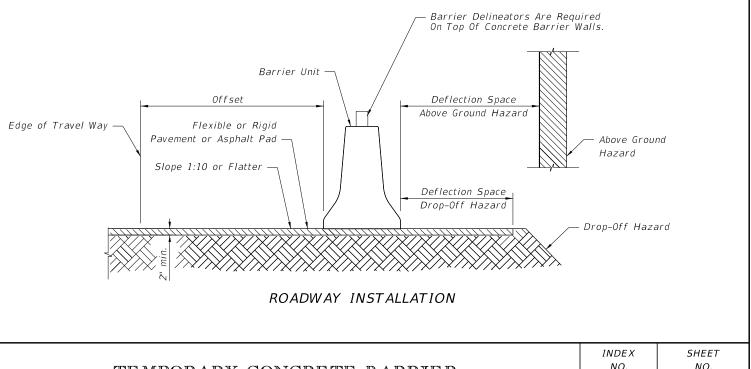




END VIEWS REINFORCEMENT AND OTHER UNIT FABRICATION DETAILS NOT SHOWN. PERMITTED BARRIER UNIT END VIEWS



MEDIAN INSTALLATION



DESCRIPTION:

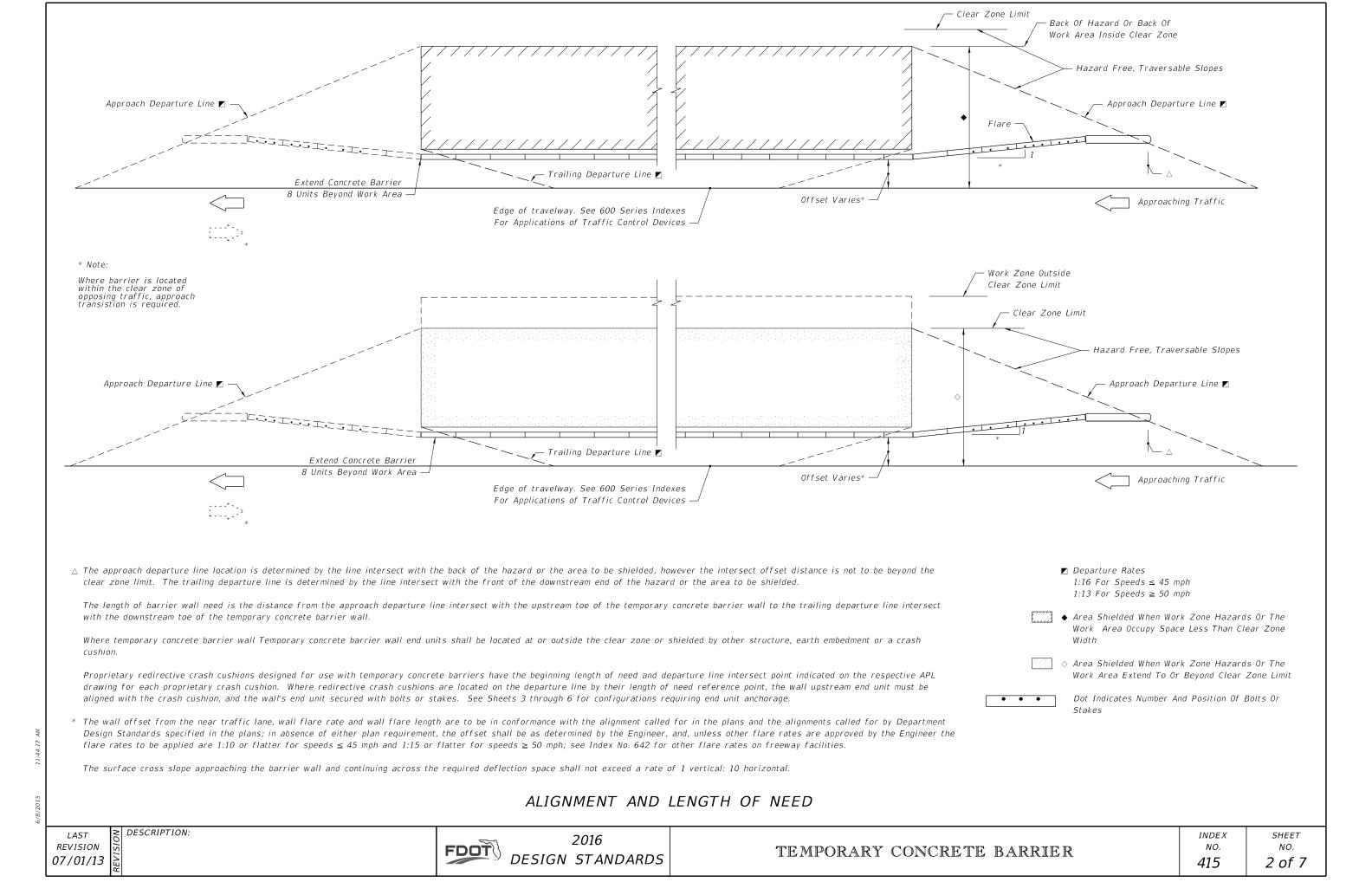
2016 FDOT DESIGN STANDARDS

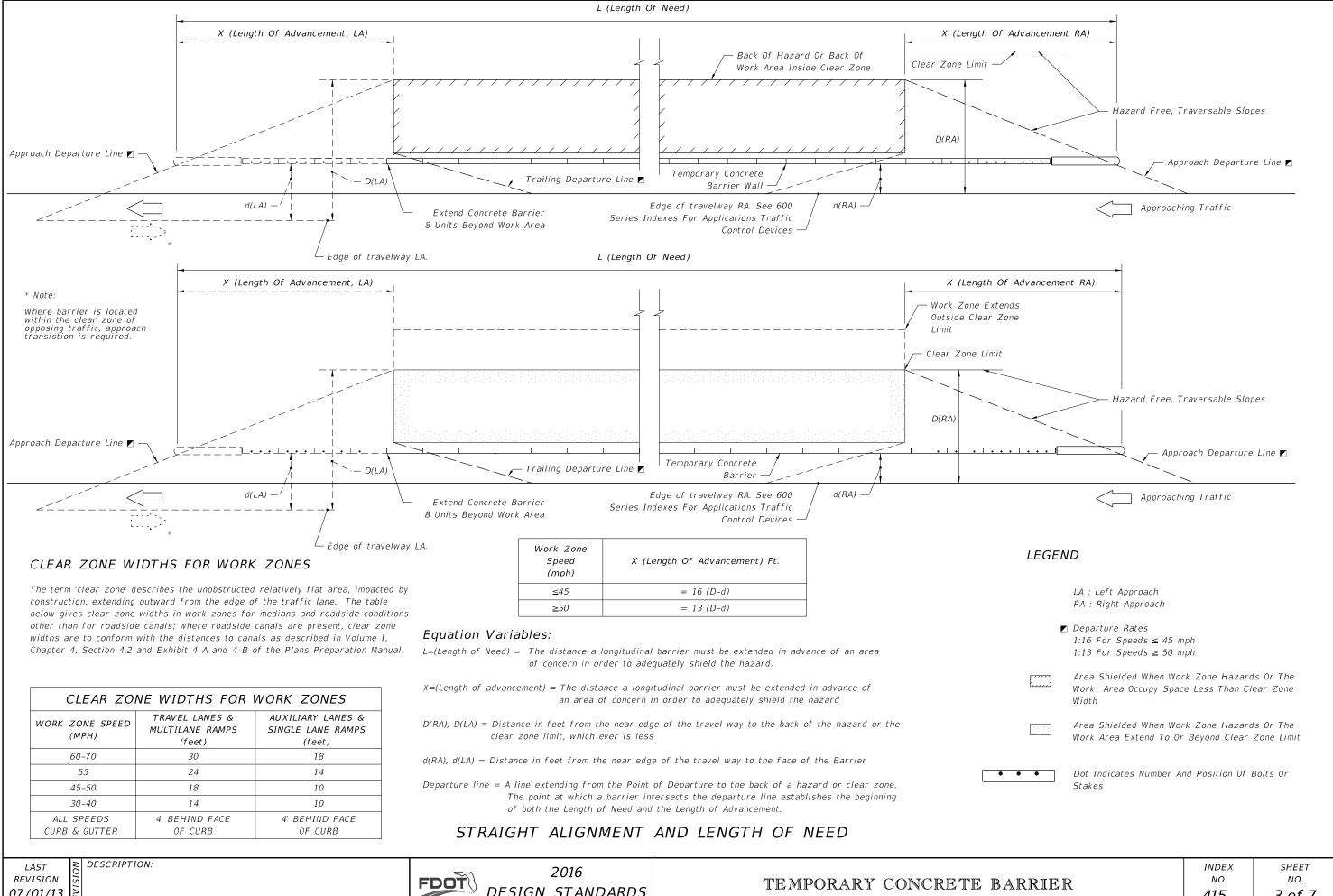
# TEMPORARY CONCRETE BARR

N.J. SHAPE

Barrier Delineators Are Required

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CLEAR ZONE WIDTHS FOR WORK ZONES				
WORK ZONE SPEED (MPH) (MPH) (feet)		AUXILIARY LANES & SINGLE LANE RAMPS (feet)		
60-70	30	18		
55	24	14		
45-50	18	10		
30-40	14	10		
ALL SPEEDS CURB & GUTTER	4' BEHIND FACE OF CURB	4' BEHIND FACE OF CURB		

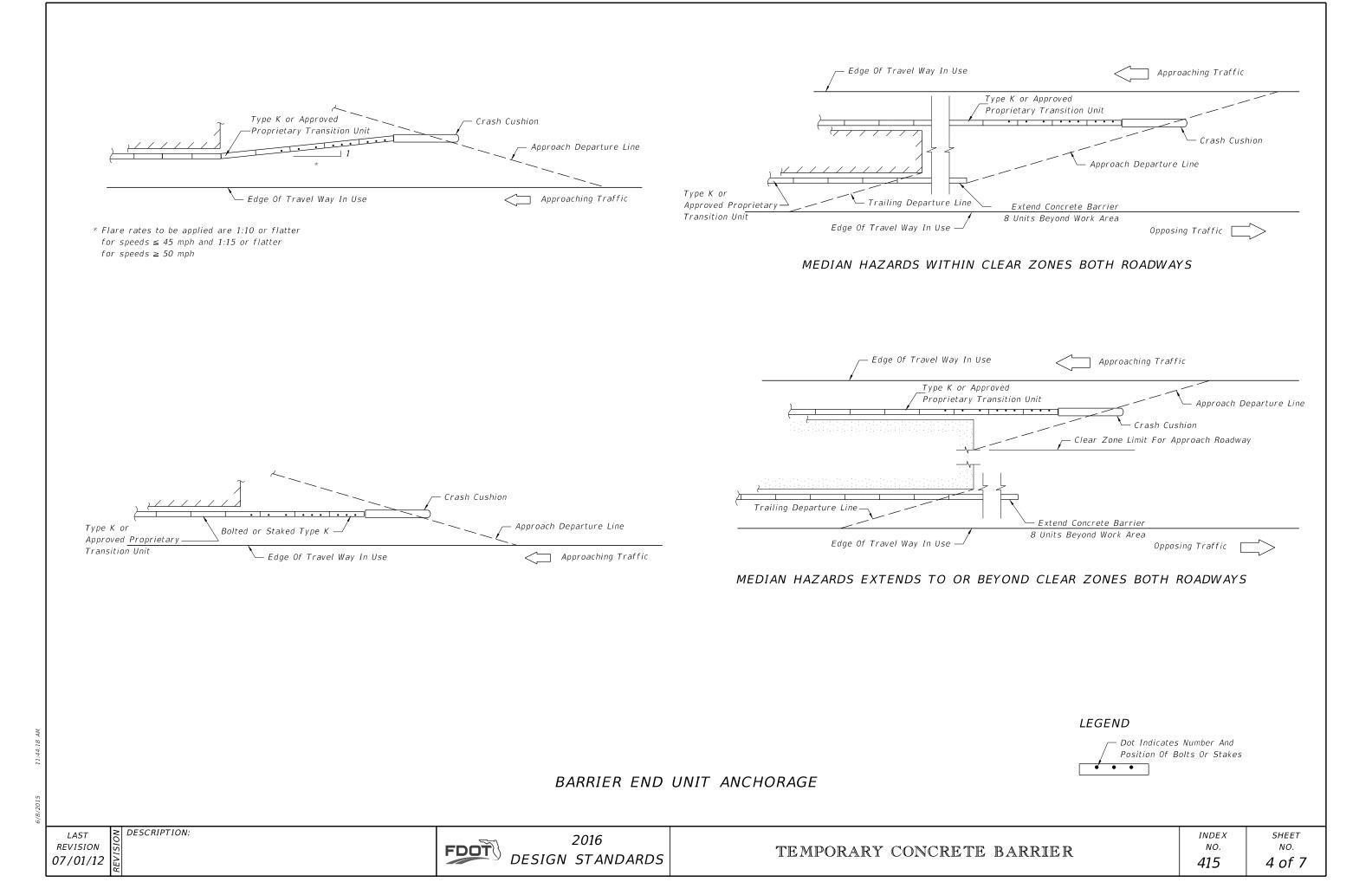
Work Zone Speed (mph)	X (Length Of Advancement) Ft.
≤45	= 16 (D-d)
≥50	= 13 (D-d)

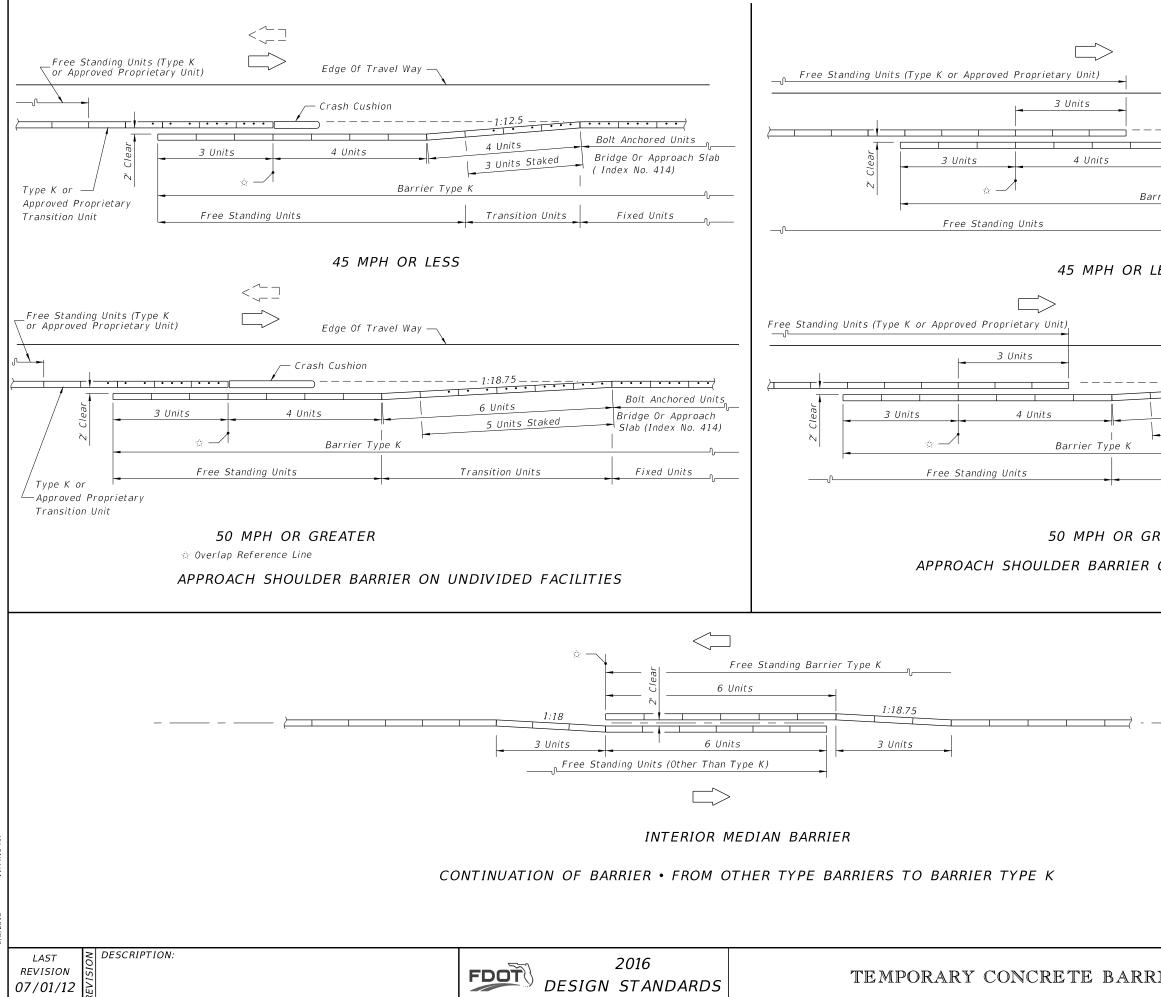
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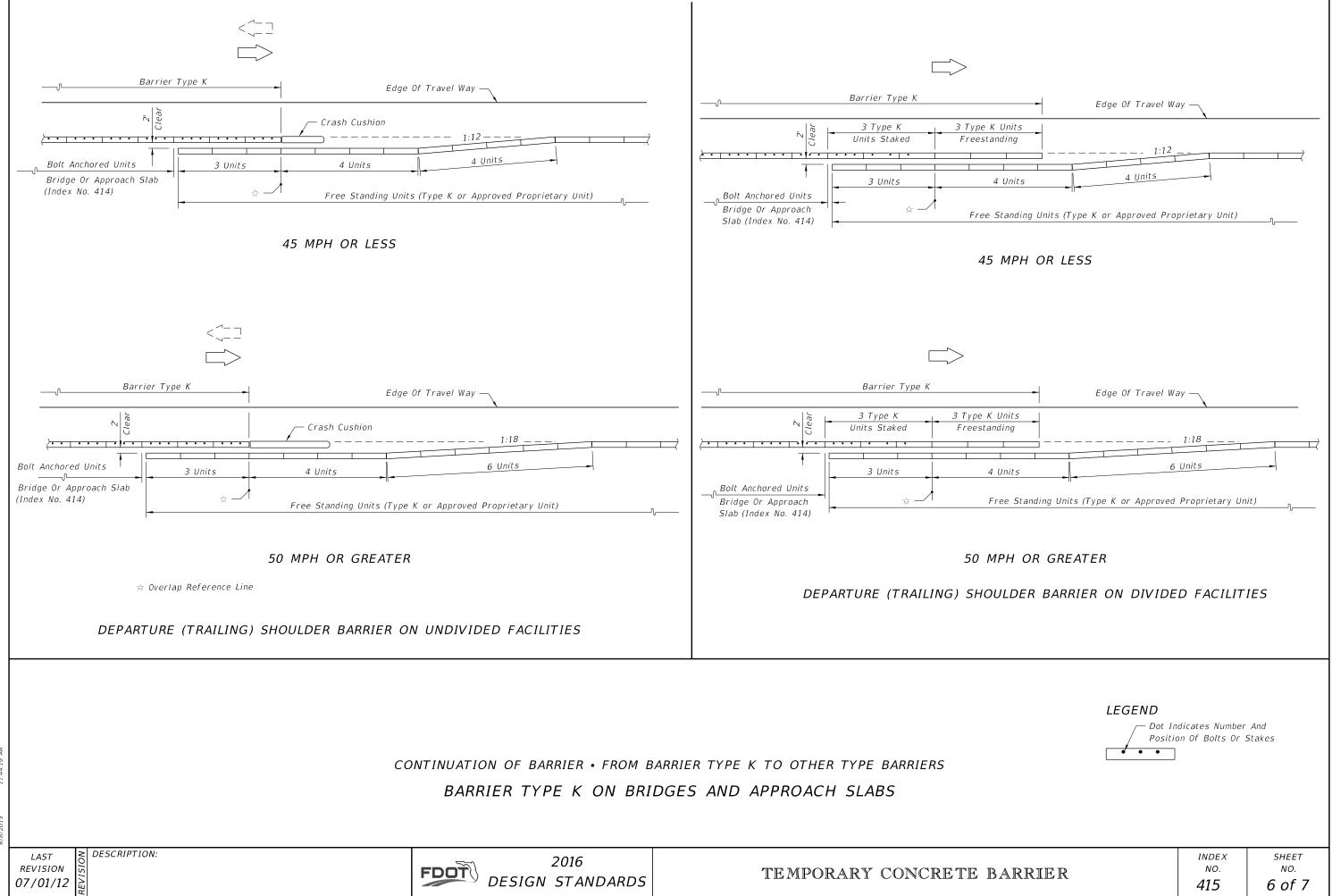
DESIGN STANDARDS

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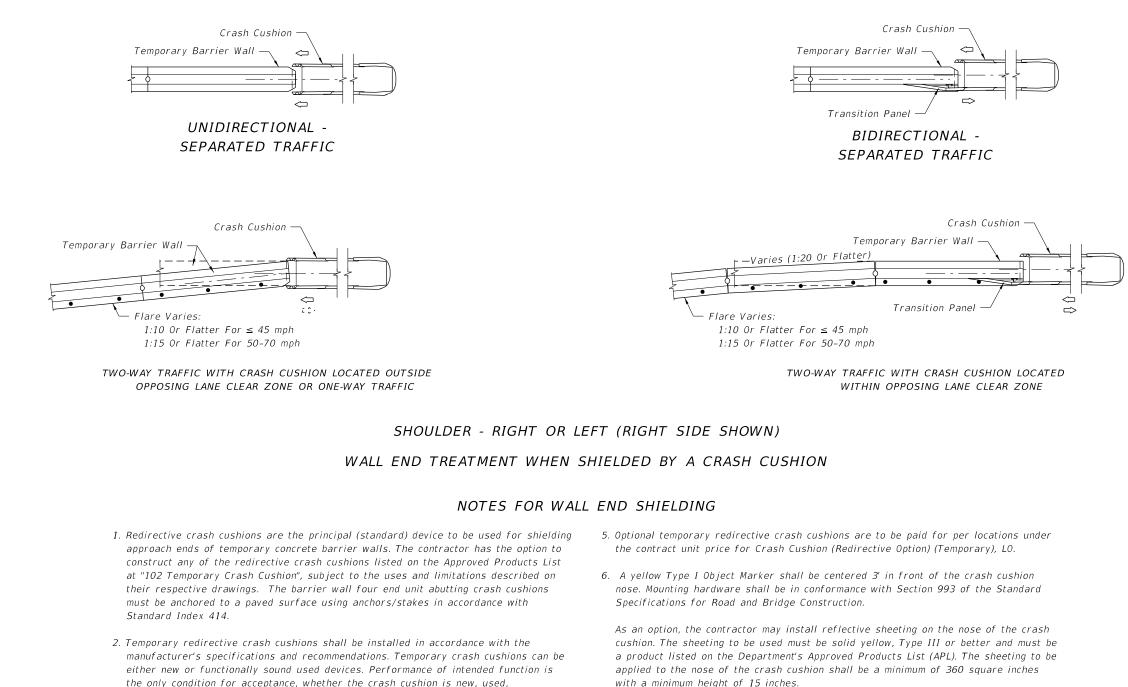




Edge Of Travel Way —		
<b>\</b>		
	Bolt Anchou Bridge Or A (Index N	pproach Slab
Transition Units	Fixed L	Jnits
ESS		
Edge Of Travel Way —		
	Bridge Or	۰۰۰۰۰۰ hored Units Approach ex No. 414)
Transition Units	Fixed	Units
REATER ON DIVIDED FACILIT	IES	
/	Indicates Number tion Of Bolts Or	
IER	INDEX NO.	SHEET NO.
	415	5 of 7



R	NO.	N
	415	60



- 7. Equipment, stockpile material, etc., shall not be placed behind the crash cushion.
- 8. When subjected to reverse direction hits, construct Transition Panels from Concrete Barrier Walls to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.
- 9. Galvanize metallic components to meet the requirements for Steel Guardrail, Section 967 of the Standard Specifications for Road and Bridge Construction.

LEGEND Dot Indicates Number And Position Of Bolts Or Stakes

DESCRIPTION:

LAST

REVISION

07/01/14

SHIELDING WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION)

2016

DESIGN STANDARDS

refurbished, purchased, leased, rented, on loan, shared between projects, or made up

3. Temporary Crash Cushions shall not be bolted down on bridge superstructures that

girders) or on bridge superstructures consisting of longitudinally prestressed,

4. Assemble and install Crash Cushions according to the limitations noted on the

Approved Products List (APL) webpage, the manufacturer's specifications, and the

FDOT

contain post-tensioned tendons within the concrete deck (top flange of concrete box

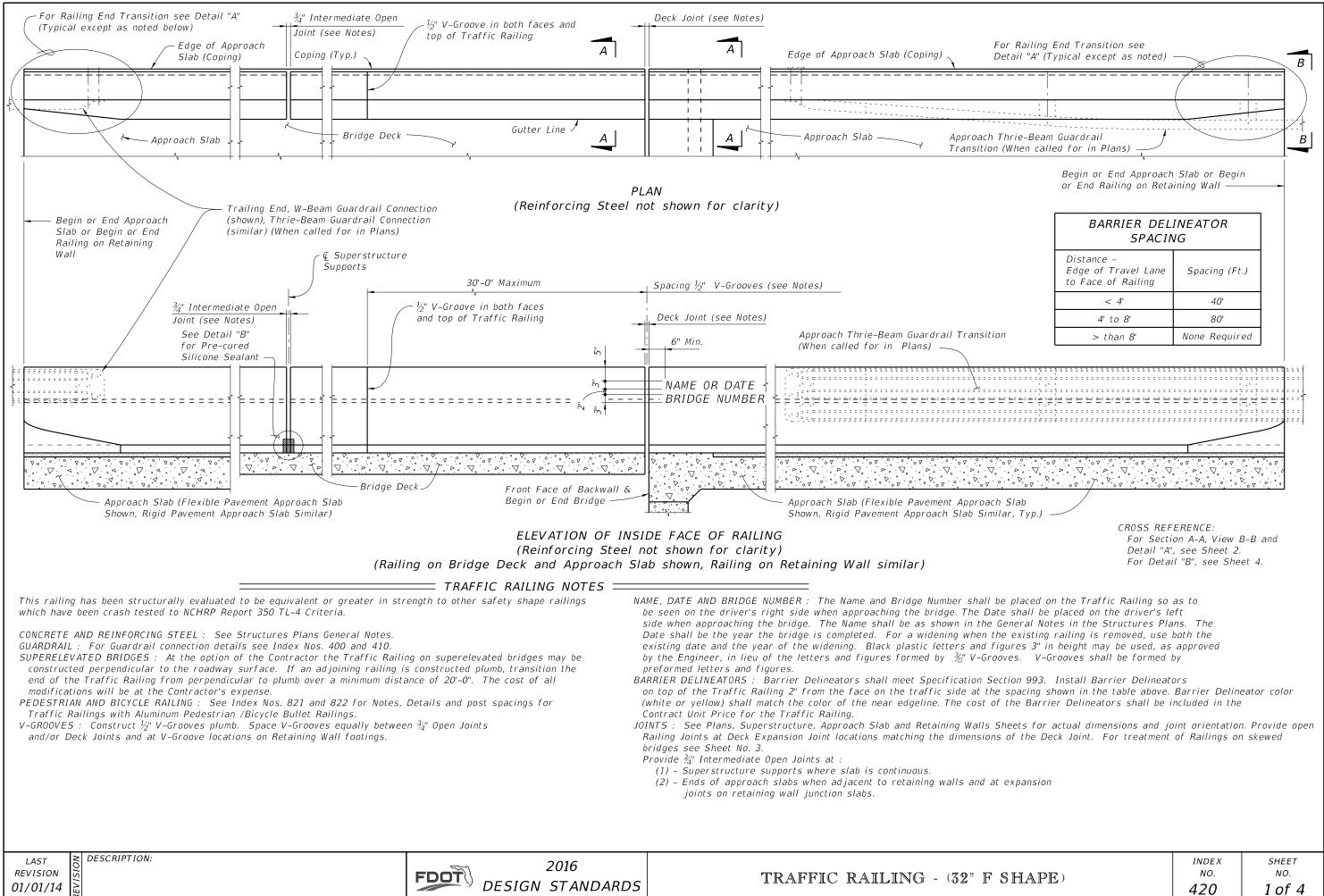
transversely post-tensioned, solid or voided concrete slab units. Gating crash cushions

of mixed new and used components.

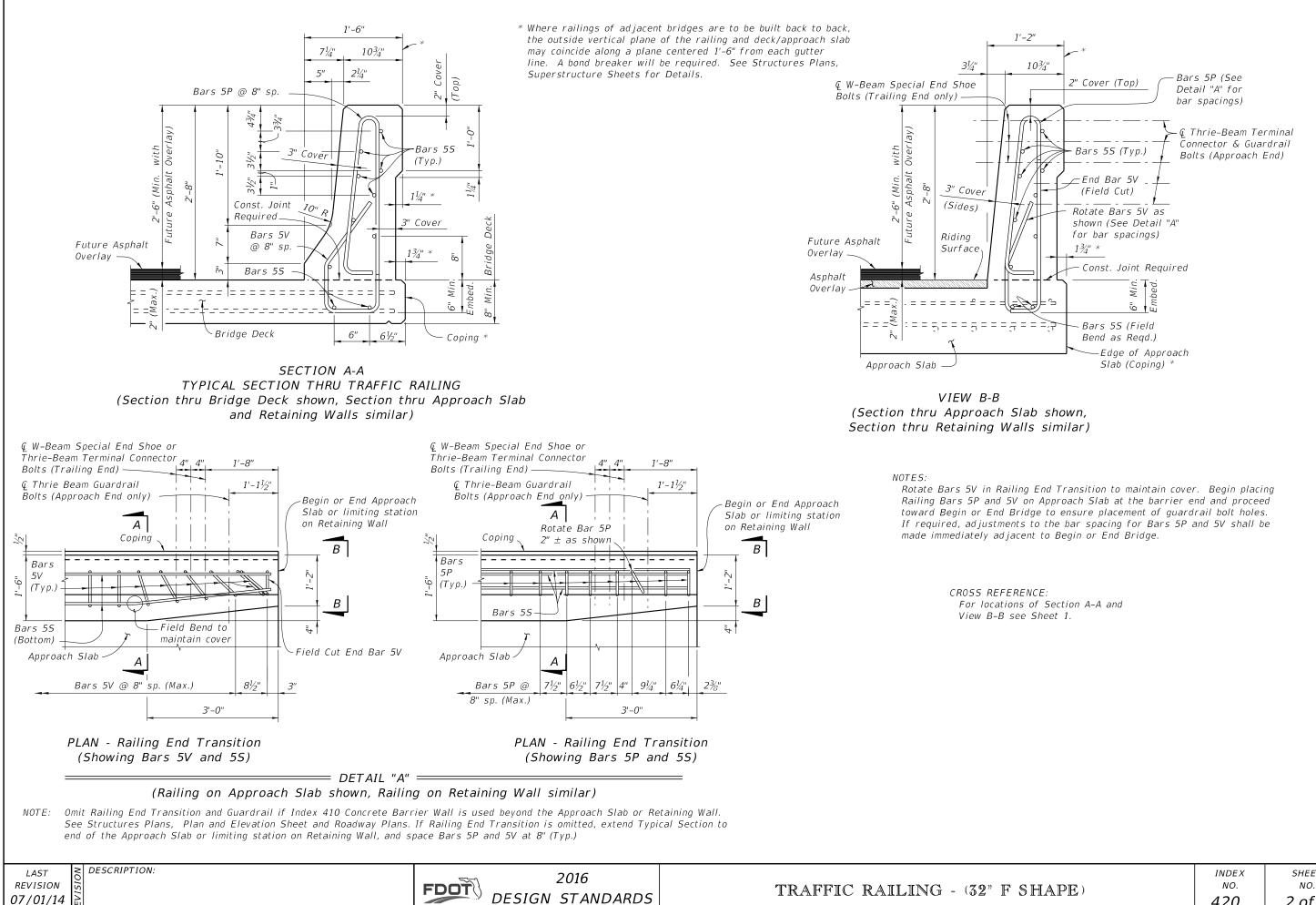
shall be used where bolting is not allowed.

applicable crash cushion drawings posted on the APL.

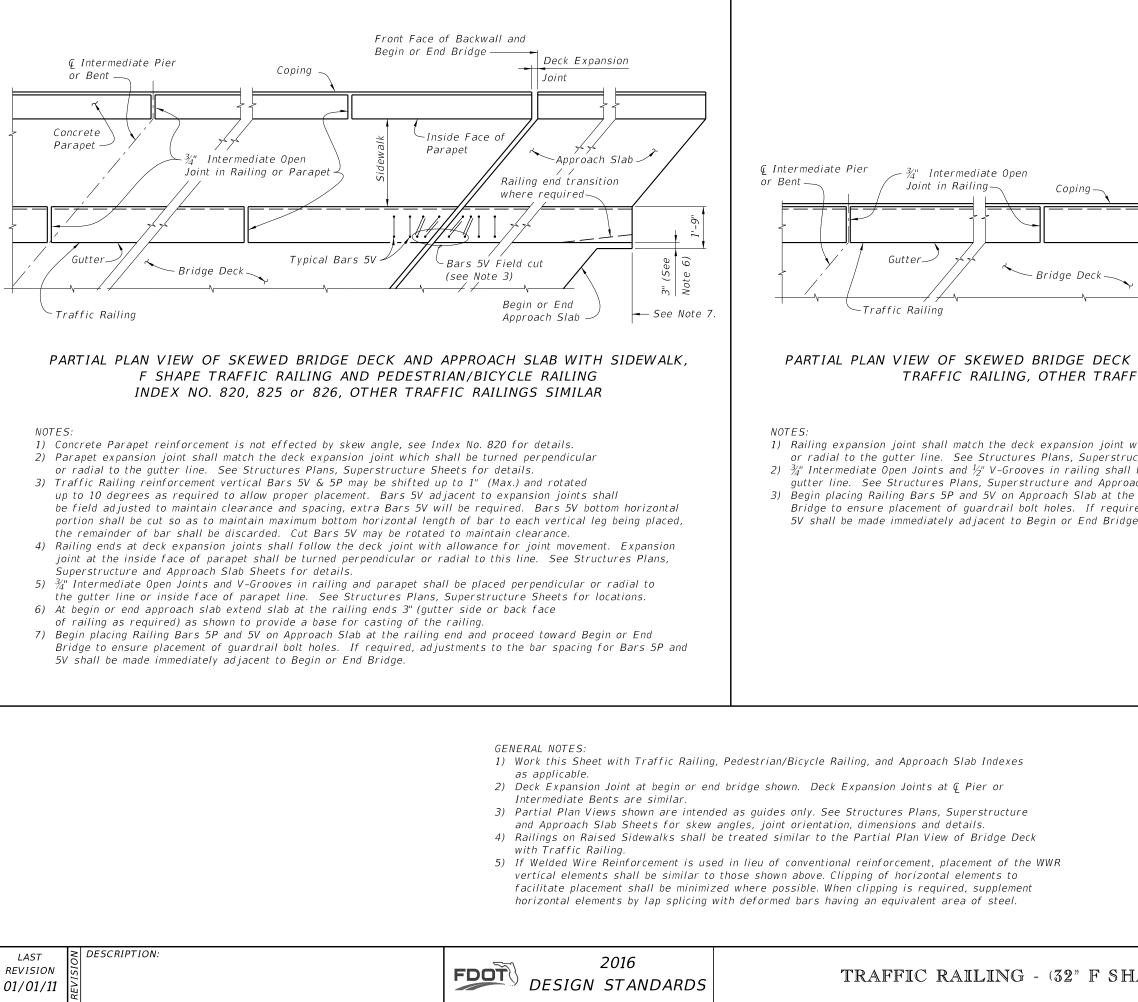
TEMPORARY CONCRETE BARRIER	index no. <b>415</b>	sнеет NO. <b>7 of 7</b>

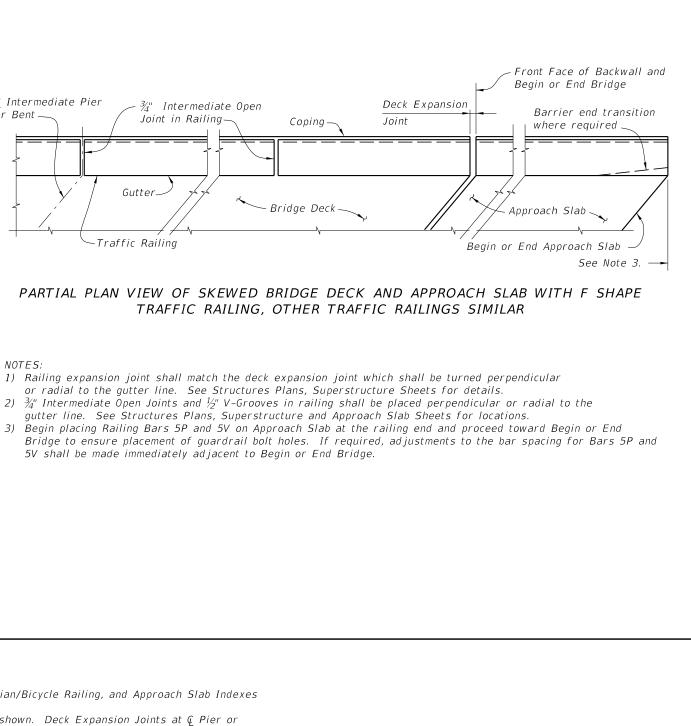






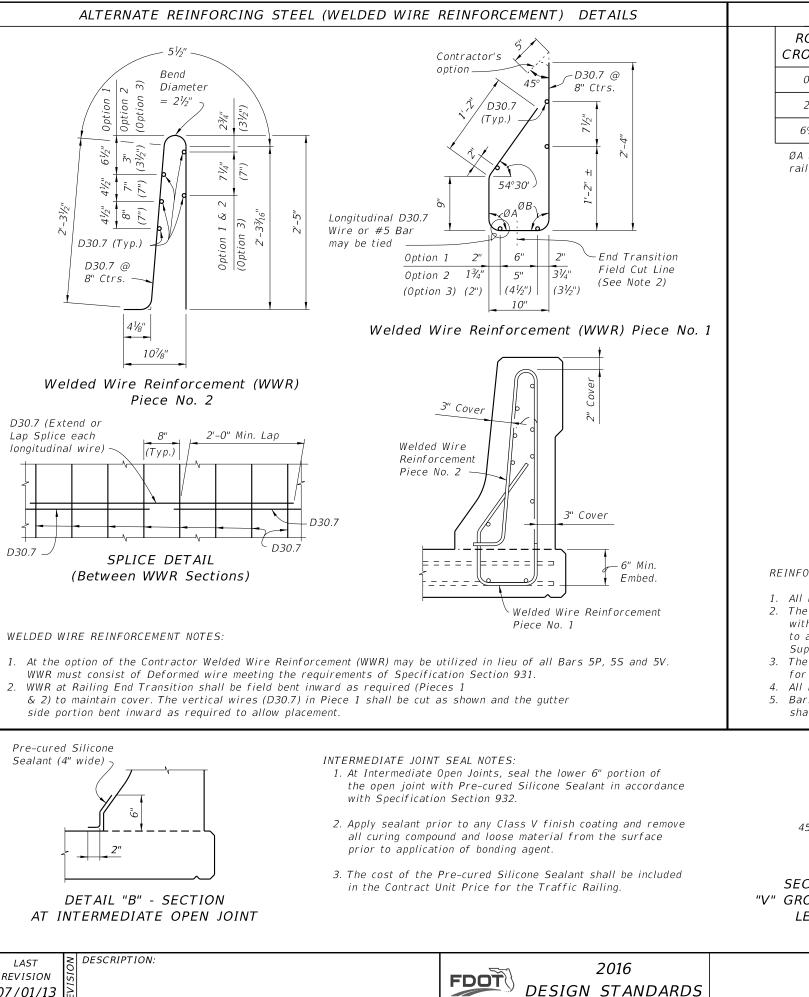
	INDEX	SHEET
APE)	NO.	NO.
	420	2 of 4





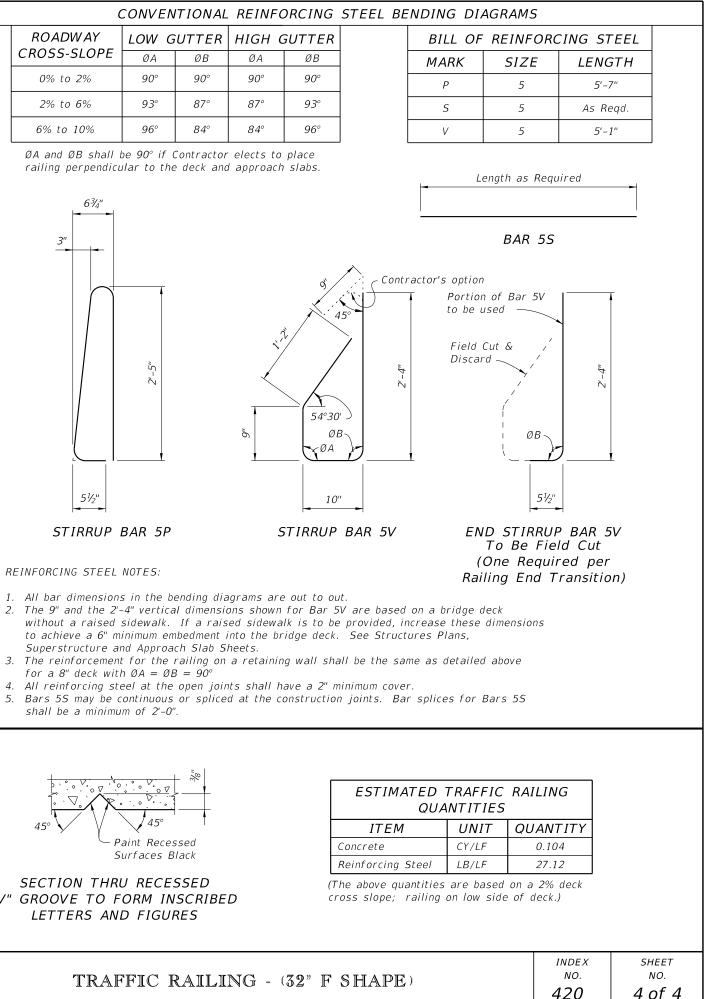
- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at Ç Pier or
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

	INDEX	SHEET
APF.)	NO.	NO.
	420	3 of 4



ROADWAY	LOW GUTTER		HIGH GUTTER	
CROSS-SLOPE	ØA	ØB	ØA	ØВ
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	9 <i>3</i> °
6% to 10%	96°	84°	84°	96°

ØA and ØB shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.



### REINFORCING STEEL NOTES:

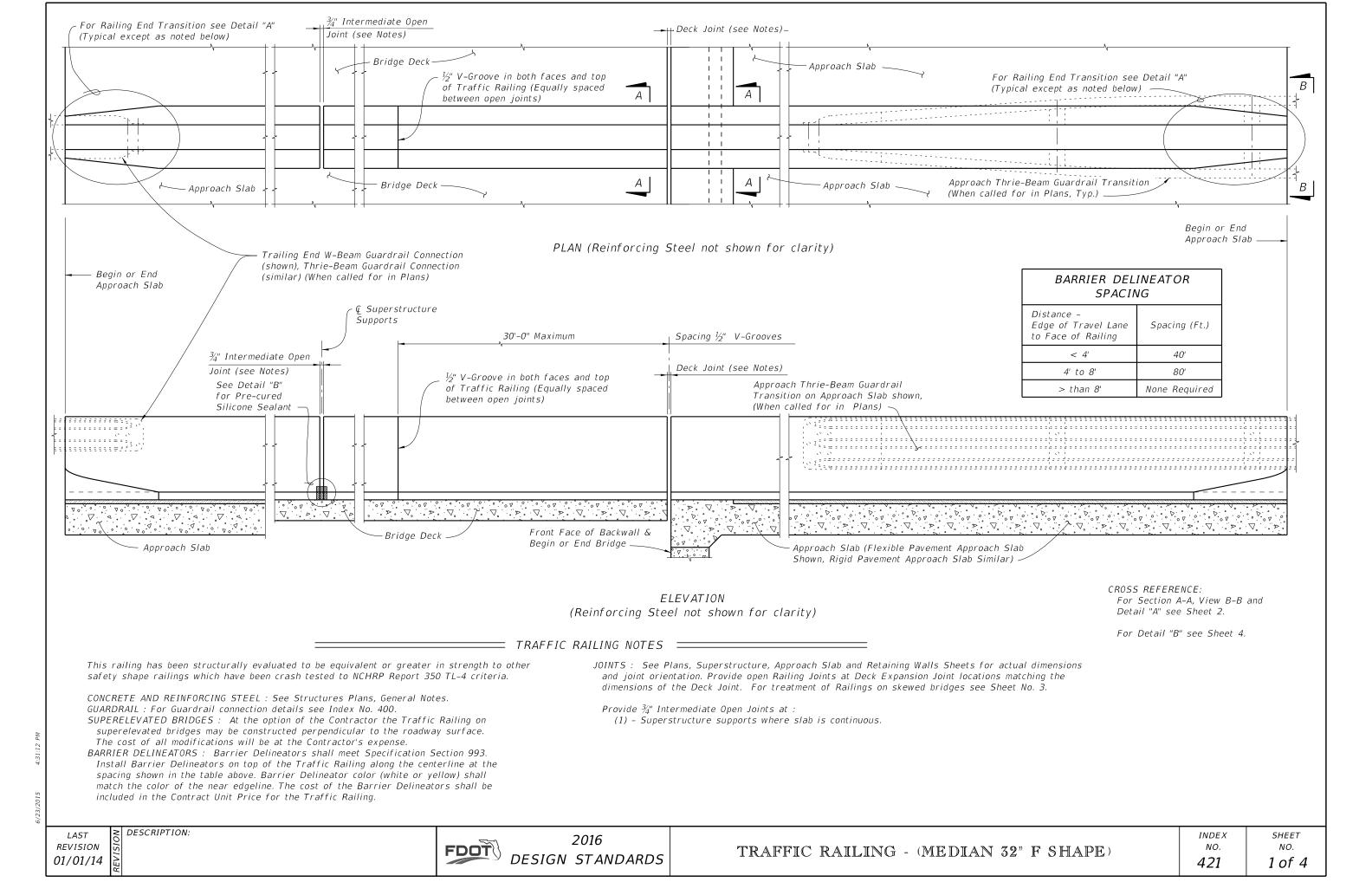
- 1. All bar dimensions in the bending diagrams are out to out.
- Superstructure and Approach Slab Sheets.
- for a 8" deck with  $\emptyset A = \emptyset B = 90^{\circ}$
- shall be a minimum of 2'-0".

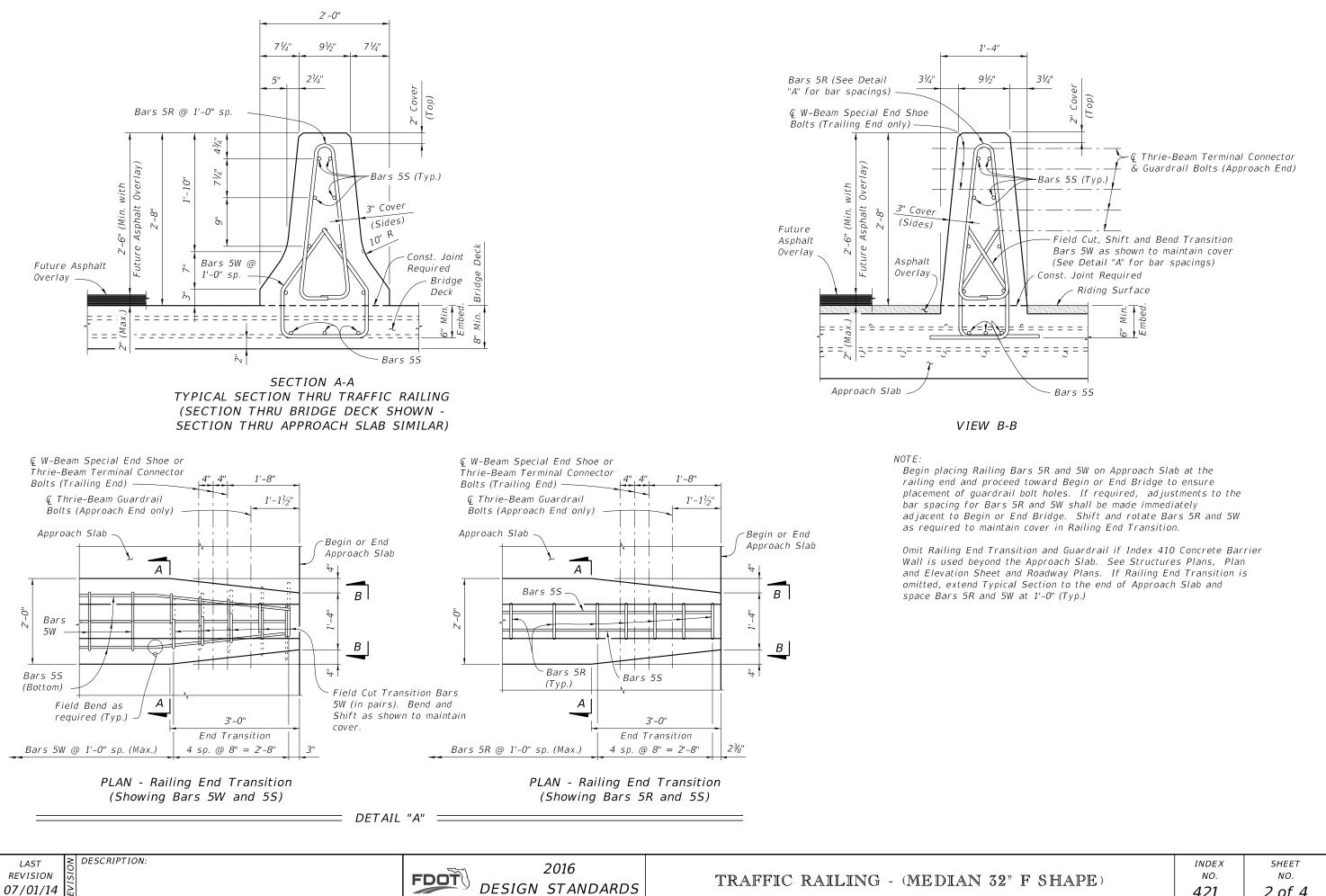
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"V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

REVISION

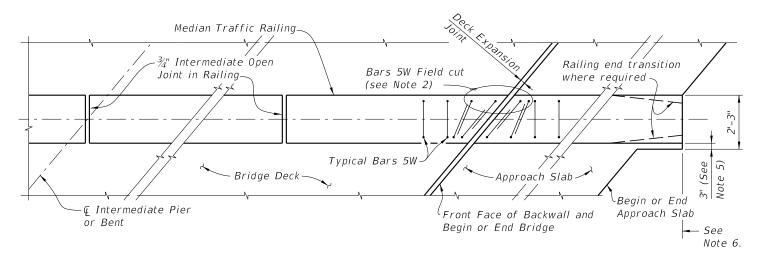
07/01/13





DESIGN STANDARDS

	INDEX	SHEET
'SHAPE)	NO.	NO.
	421	2 of 4



## PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

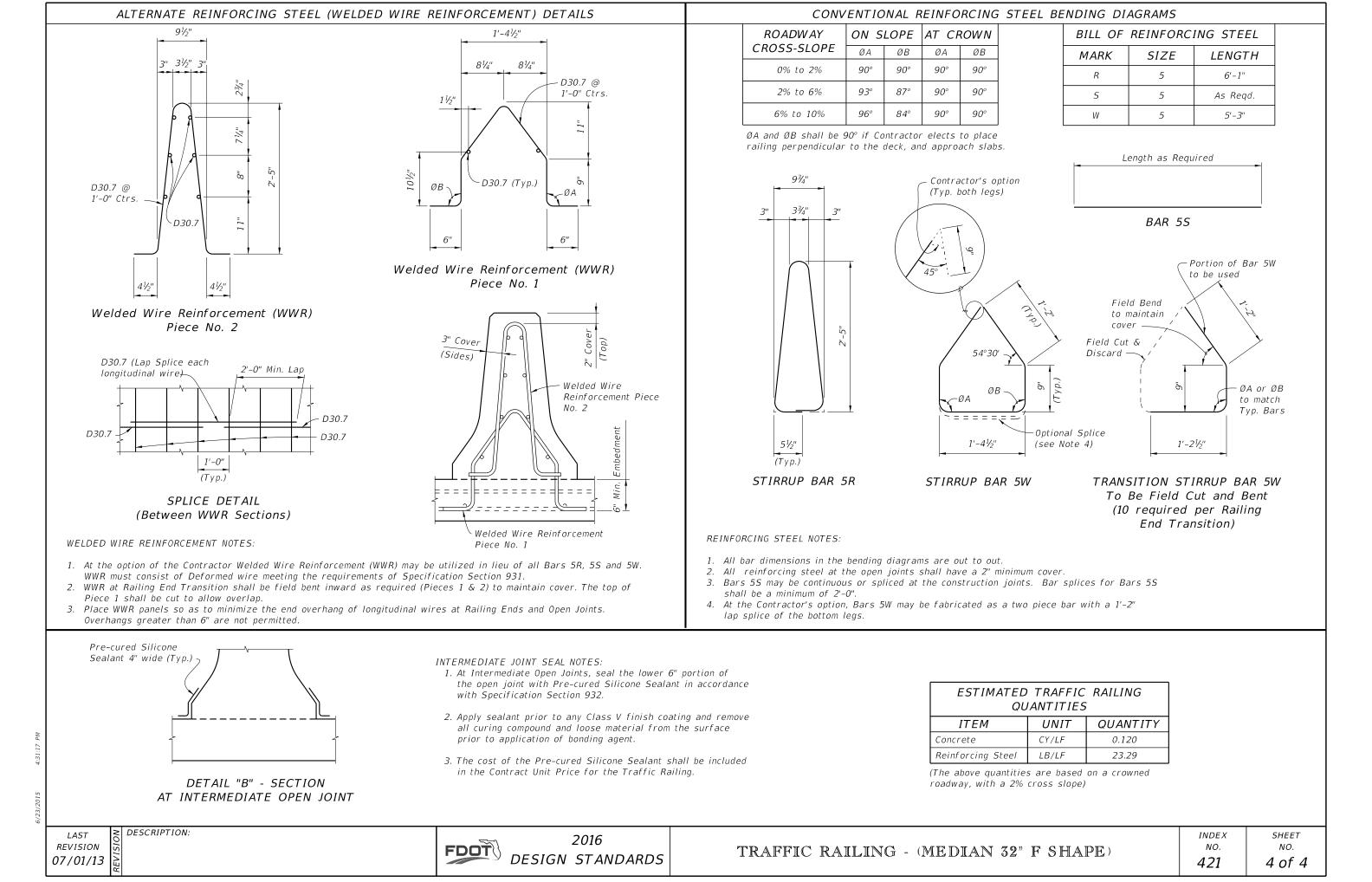
### NOTES:

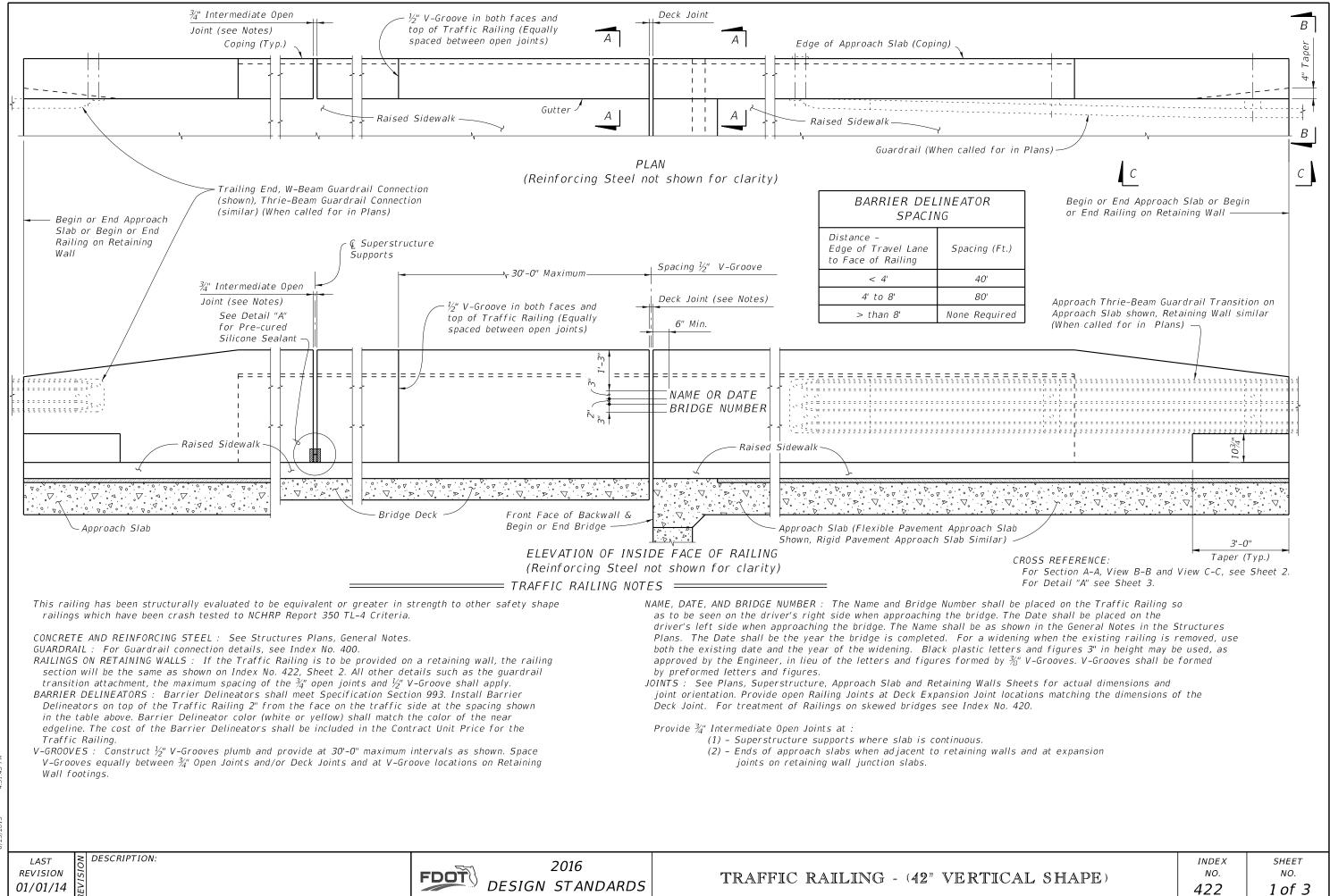
- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at *Q* Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing deformed bars with an equivalent area of steel.

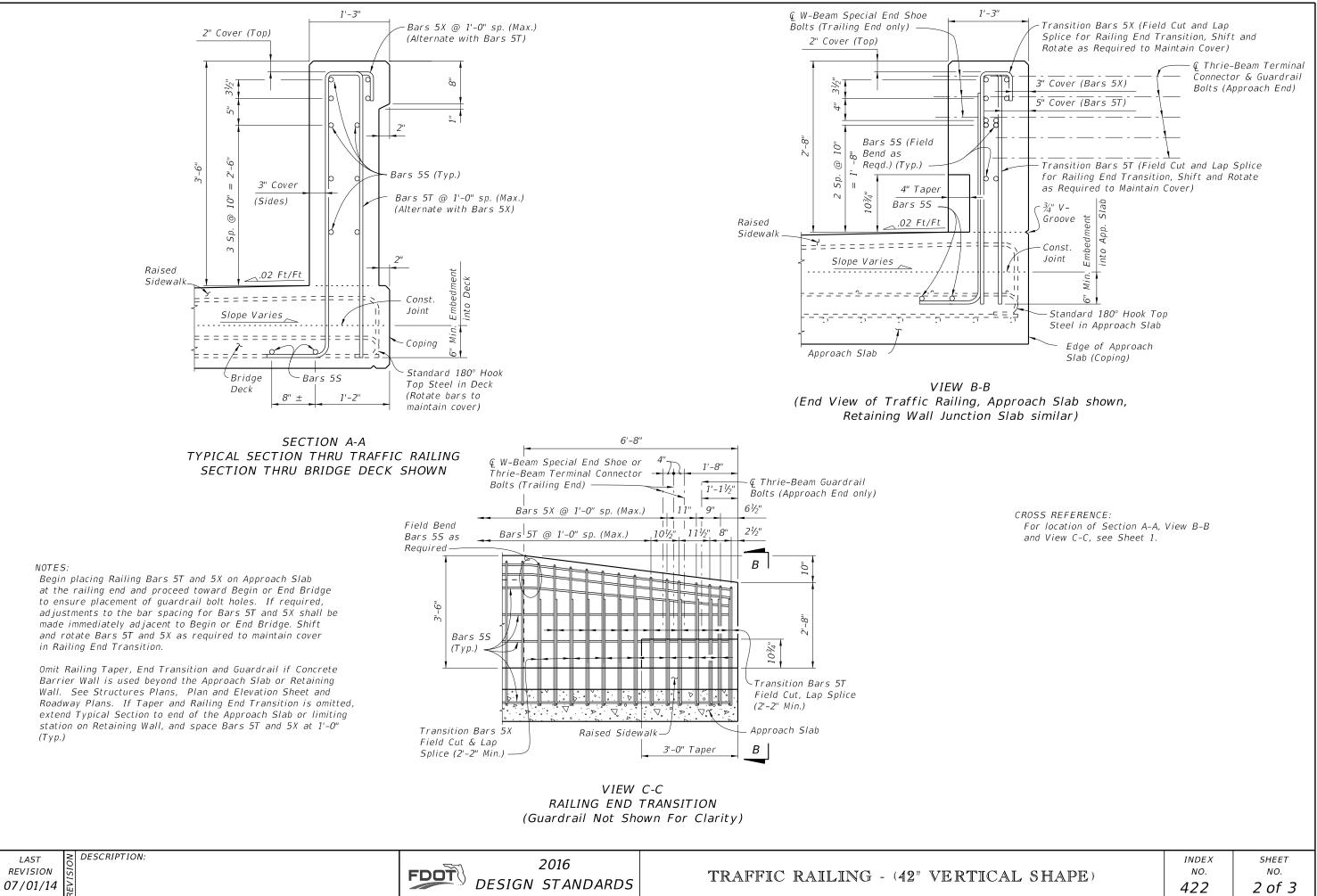


TRAFFIC RAILING - (MEDIAN 32" F

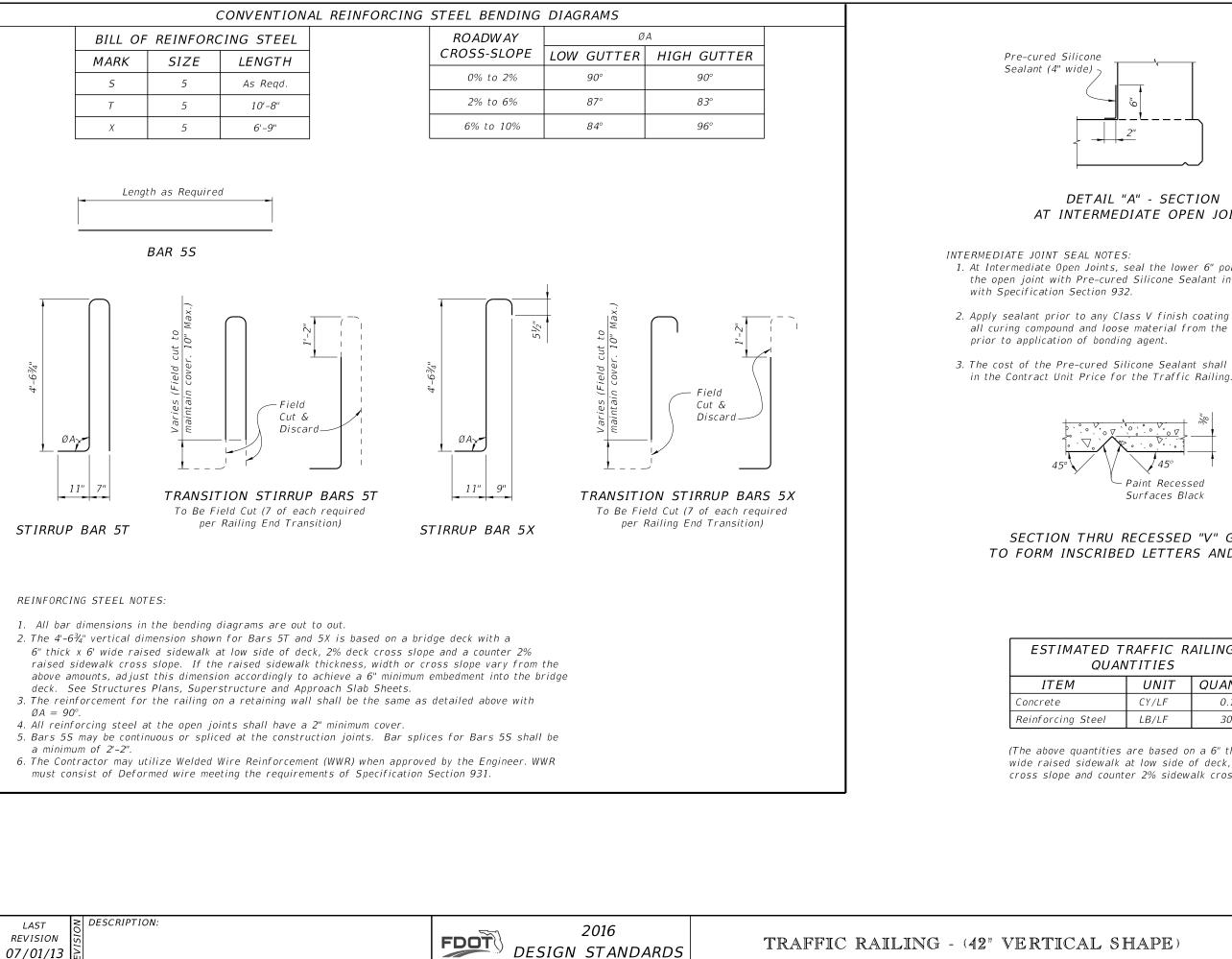
SHAPE)	index NO. <b>421</b>	<sup>sнеет</sup> NO. <b>3 of 4</b>

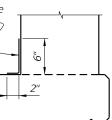






	INDEX	SHEET
(SHAPE)	NO.	NO.
	422	2 of 3



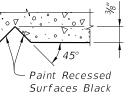


## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance

2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface

3. The cost of the Pre-cured Silicone Sealant shall be included

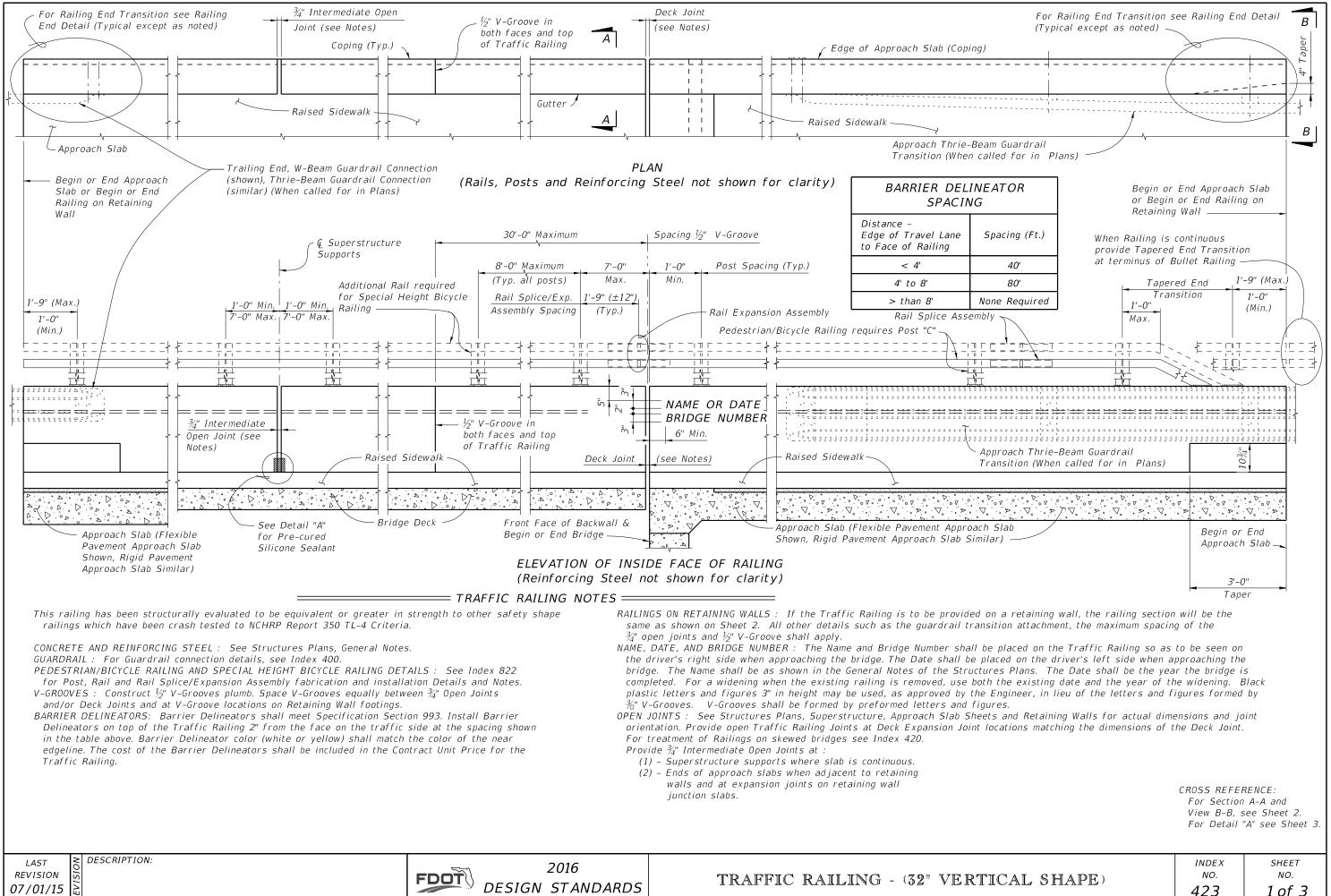


### SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

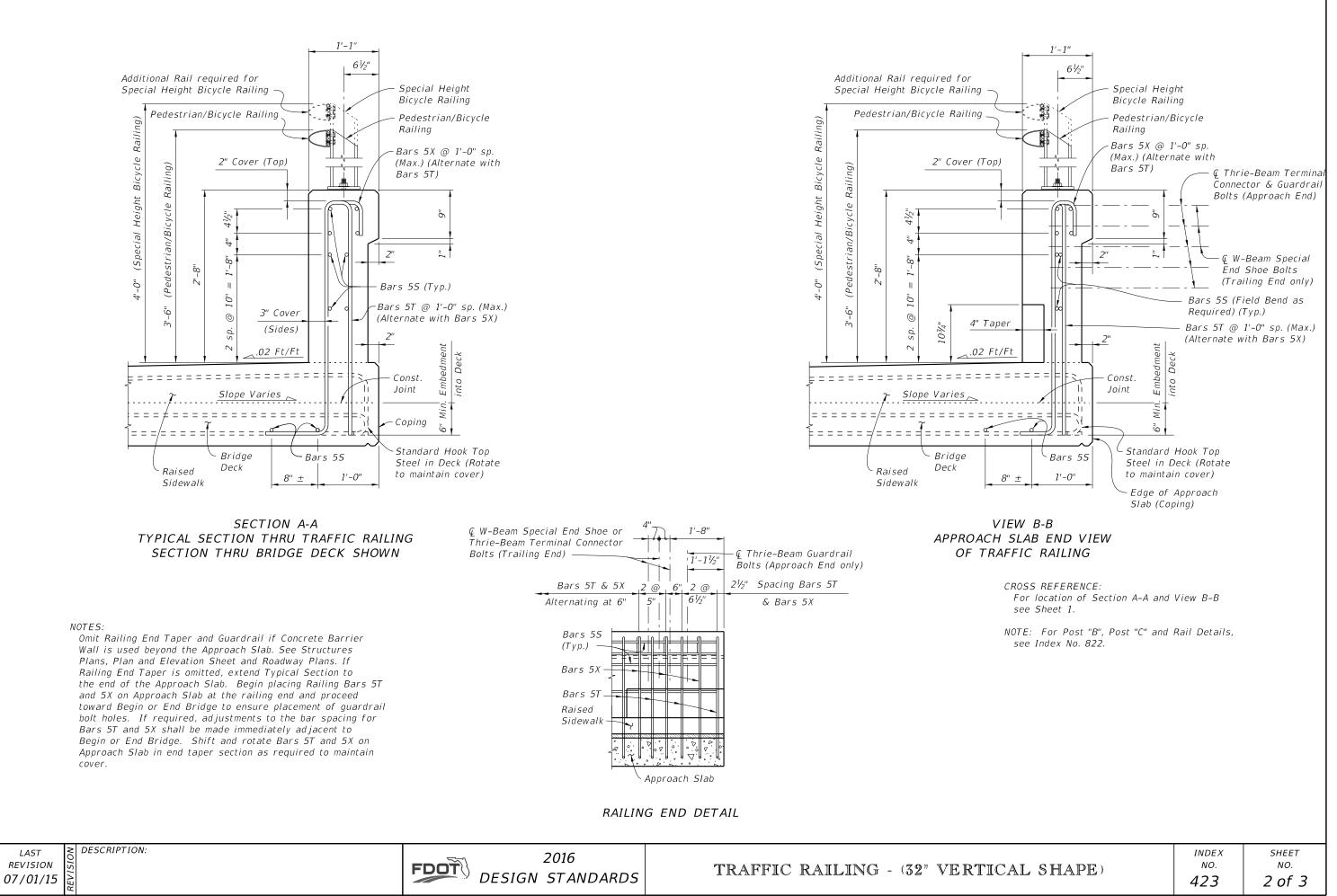
D TRAFFIC RAILING DUANTITIES		
	UNIT	QUANTITY
	CY/LF	0.145
el	LB/LF	30.68

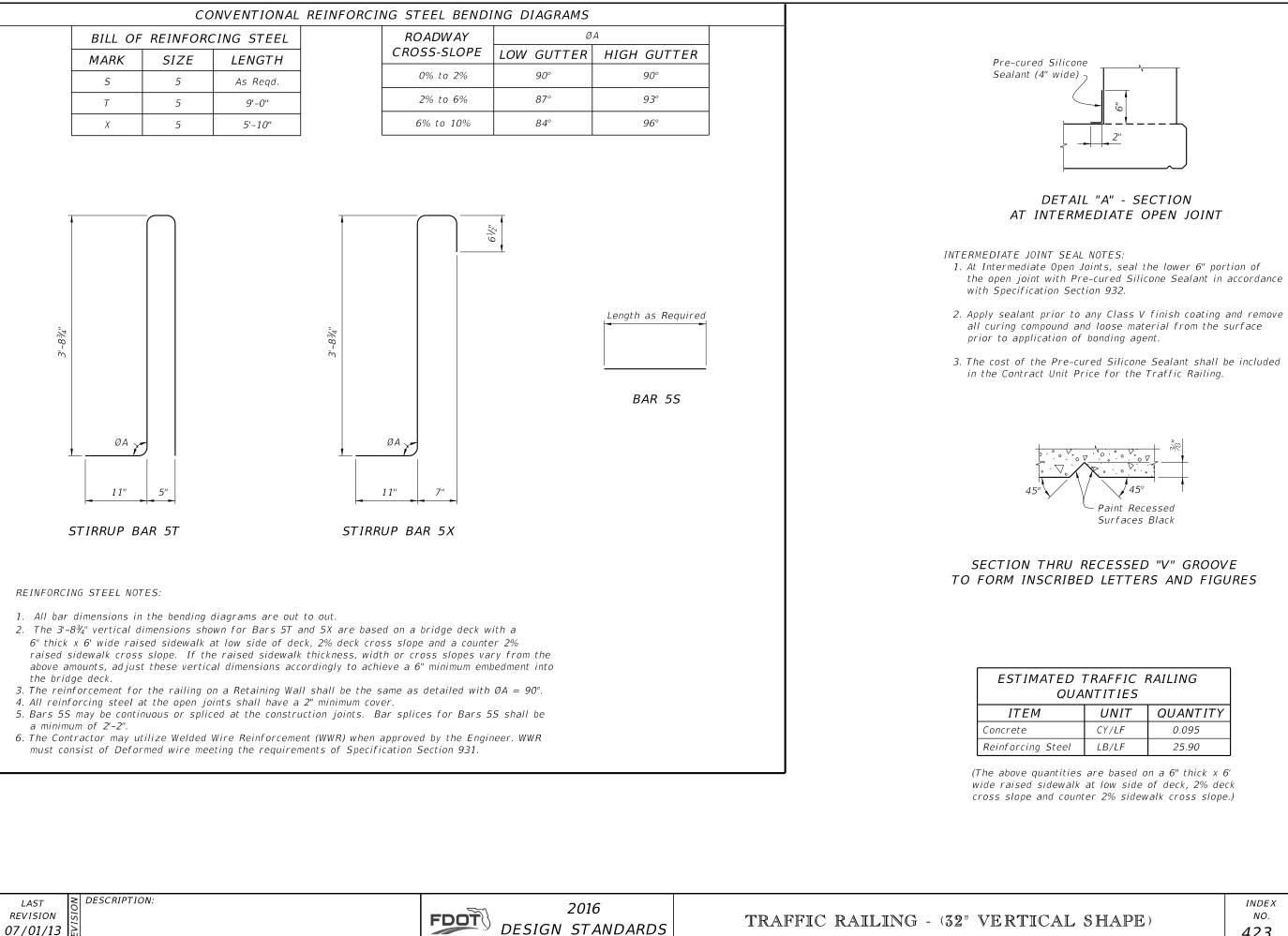
(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)

	INDEX	SHEET
(SHAPE)	NO.	NO.
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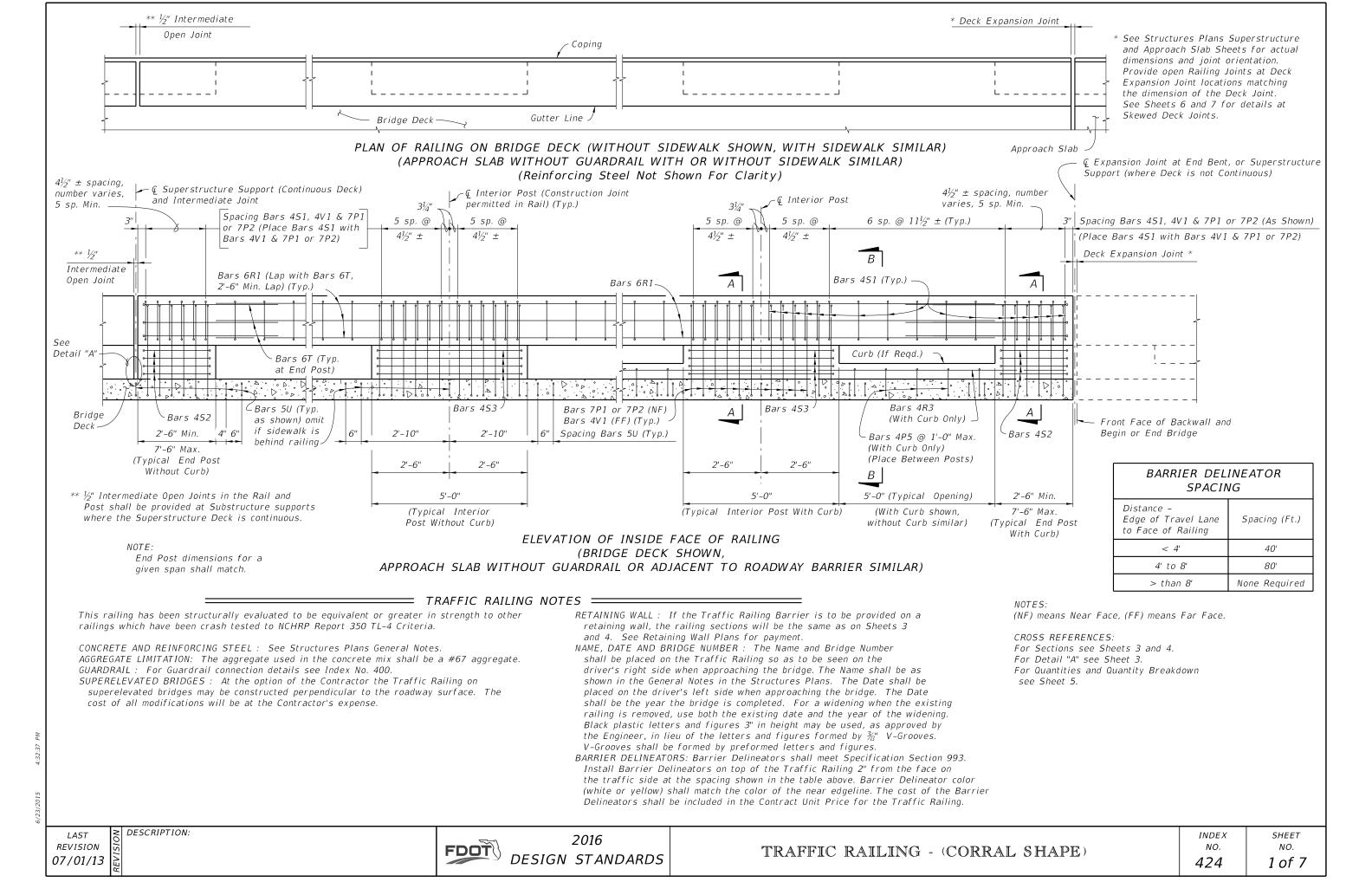
4:32	
2015	

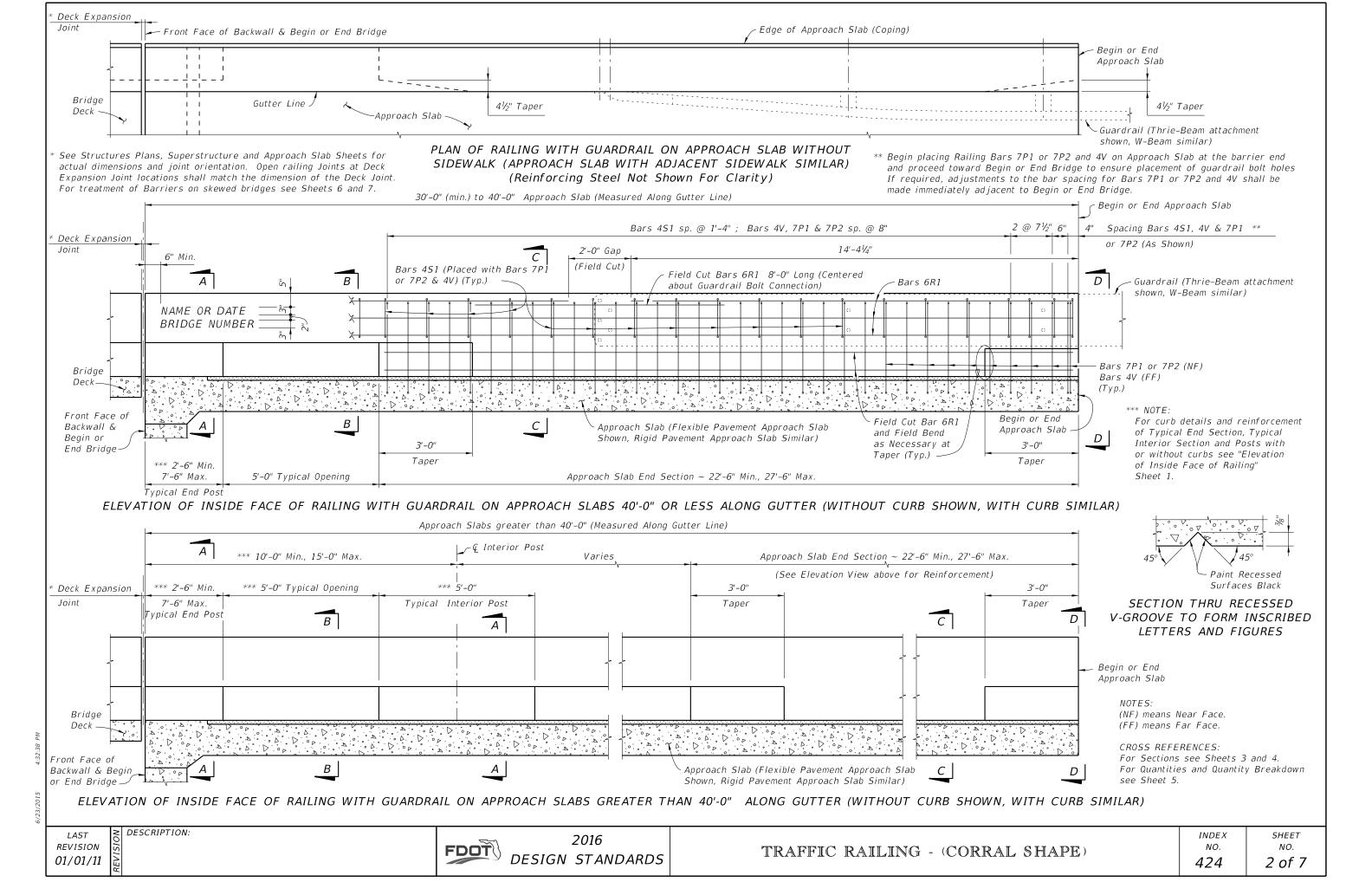


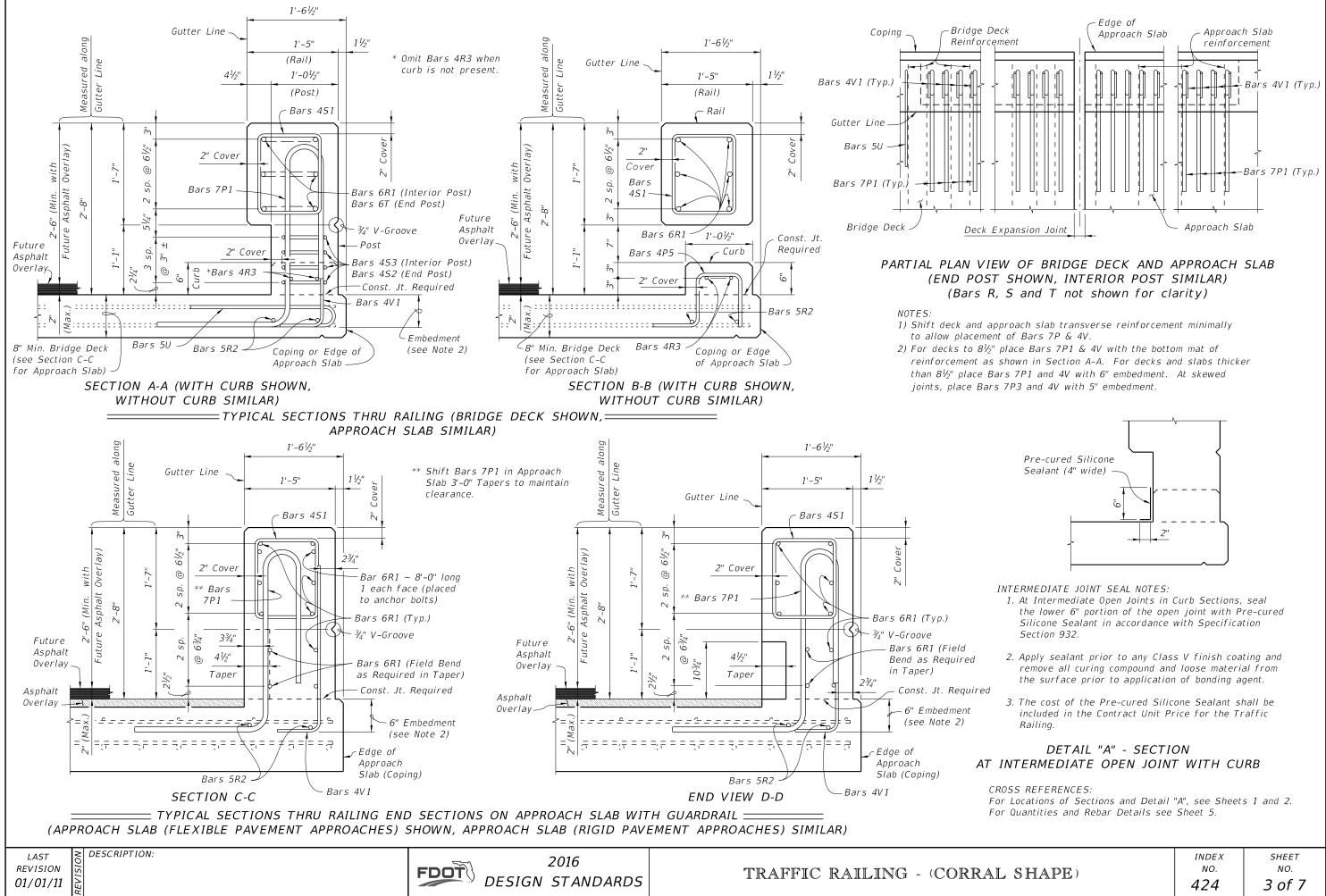


 TRAFFIC I NTITIES	RAILING
UNIT	QUANTITY
CY/LF	0.095
LB/LF	25.90

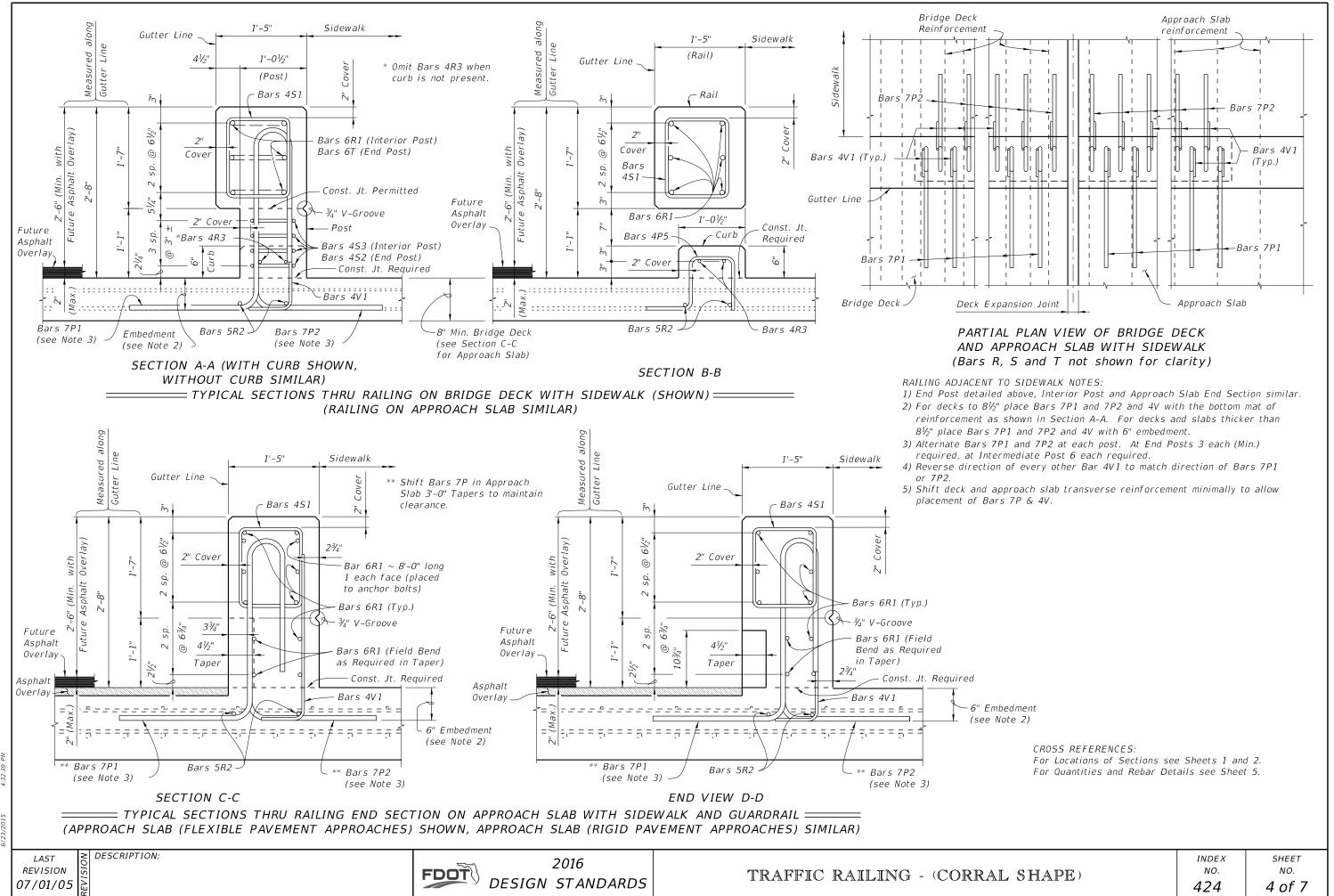
SHAPE)	INDEX NO.	SHEET NO.
	423	3 of 3





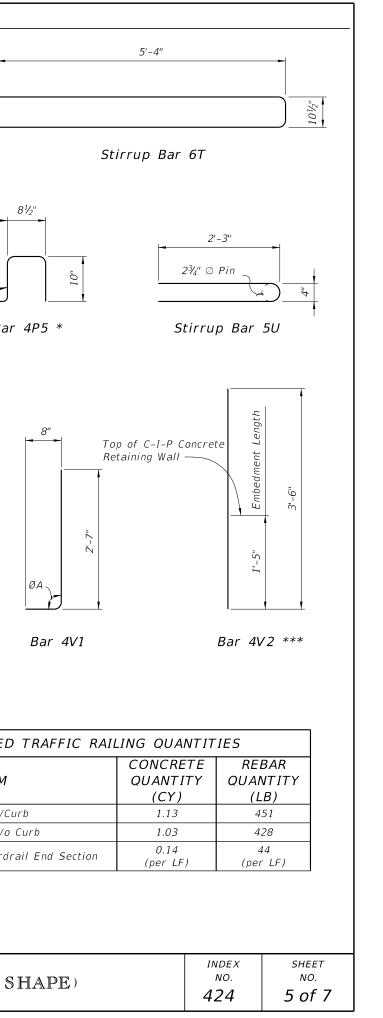


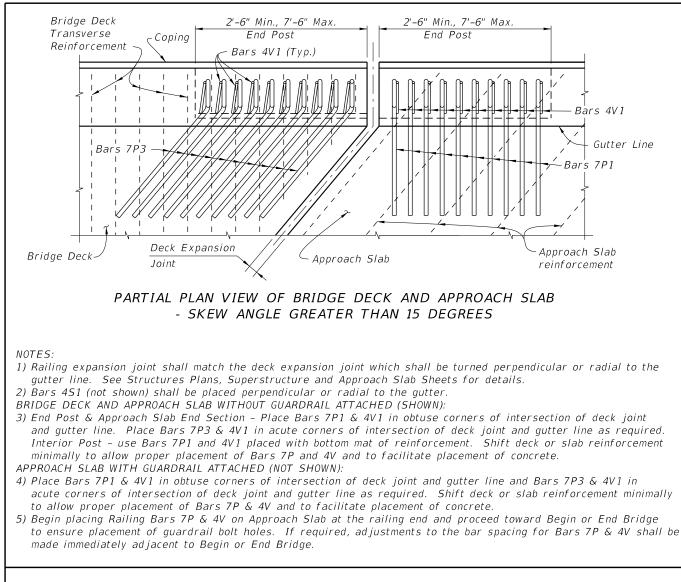
	INDEX	SHEET
HAPE)	NO.	NO.
	424	3 of 7

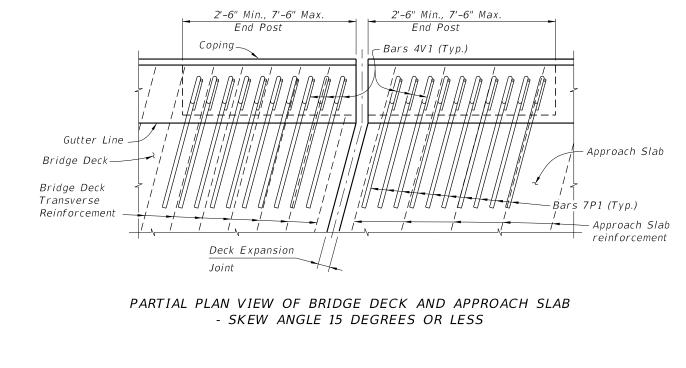


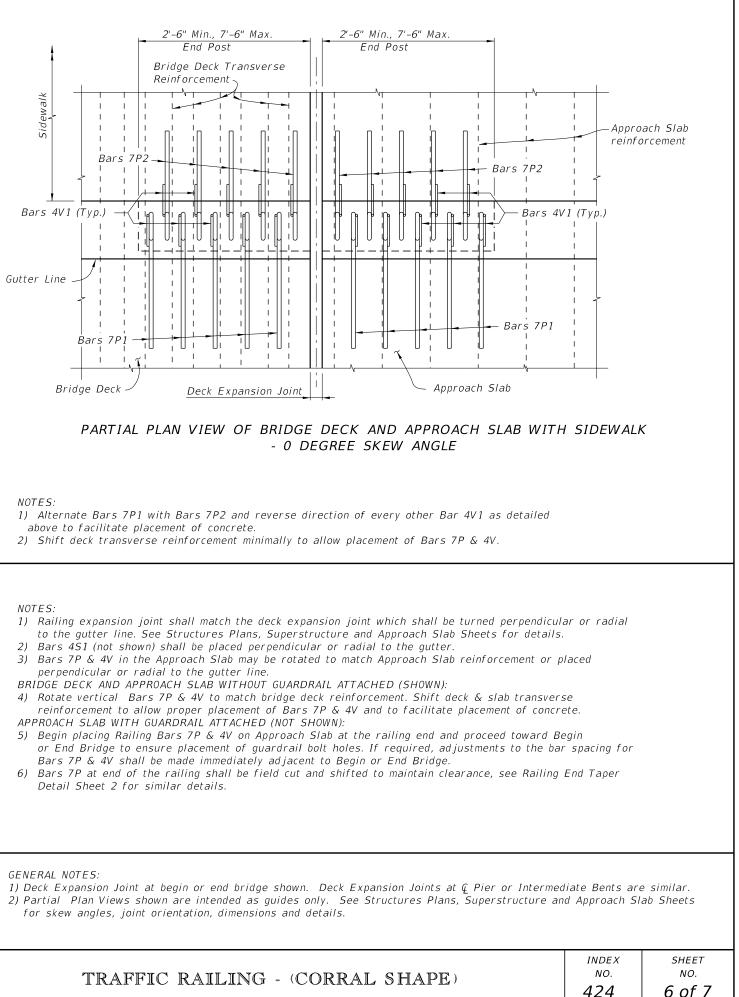
	INDEX	55551
F.)	NO.	NO.
( <u>L</u>	424	4 of 7

					CONVENTIC	NAL REINFORC	ING STEEL E	BENDING DIAGRAMS	5	
BILL OI	F REIN	FORCING	STEEL		11 01	8 <sup>1</sup> /2"	1	4'-8"		
MARK	SIZE	LENGTH	LB/BAR		1'-0"		-			-
P1	7	7'-4"	15.00							
P2	7	7'-3"	14.82	Bars 6R1, 5R2 & 4R3 *		ľ.	Min.		ļ	81/2"
Р3	7	7'-2"	14.65	Length As Required	1,-3"		<sup></sup>		)	
*** P4	7	7'-3''	14.82				<u>7'-2"</u>	Stirrup B	ar 453	
* P5	4	2'-11"	1.94				Varies 7'-			
R1	6	As Reqd.	1.5 (LB/LF)	Bars 6R1, 5R2 & 4R3	Stirrup Bar 4S1	$\bigcup_{-}$				8"
R2	5	As Reqd.	1.04 (LB/LF)			Stirrup Bar	160		and c	
* R3	4	As Reqd.	0.67 (LB/LF)			Stirrup Bar 4	452	2'-1''		
** 51	4	5'-0''	3.34					2'-1		ØA
** 52	4	Varies 6'-3" Min. 16'-3" Max.	Varies 4.18 Min. 10.86 Max.		-	2'-0"		Ī	5-1"	
** 53	4	11'-3"	7.52							Bar
Т	6	11'-4"	17.02					=		
U	5	4'-8''	4.87					2. - 9.		
V 1	4	3'-2''	2.12	-10,	2'-1"	2'-1'				
*** V2	4	3'-6"	2.34		2-1			Parallel to Joint	ØA J-	
	94 <sup>°</sup> & 4V2 e Retaini.		sed on C-I-P	Bar 7P1	Ba	nr 7P2	Structures F Superstructu			
		EL NOTES: ons in the be	ending diagrams	are out to out.	SIDEWALK S	IGH LOW IDE SIDE ØA ØA				
2. The rei	nforceme	ent for the r	ailing on a C-I-	-P Concrete Retaining Wall 8" deck with ØA = 90°,	0% to 2%	90° 90°		- 1-		
where	applicabl	e. If bottom	n horizontal leg	s of Bars 7P1, 7P3 and 4V1 be substituted for Bars 7P1,	2% to 6%	93° 87°	Top of C-I-P	5,		
7P3 an	d 4V1 as	shown.		nall have a 2" minimum cover	6% to 10%	96° 84°	Concrete Retaining		· · · · · · · · · · · · · · · · · · ·	
unless	otherwis	e noted.		4R3 may be continuous or	ØA shall be 90° if Contr	actor elects	Wall			ESTIMATED
spliced	. Where	bars are sp	oliced provide a	2'-6" Min. lap length for	to place Railing Perpent the Deck.	dicular to		th 4'		ITEM
length	for Bars	4R3.	-	5R2 and a 1'-3" Min. lap				Lengi		
				om joint to joint and side ure Sheets for details.				2'-6" Embedment Length		D'-O" Section w/Cu D'-O" Section w/o
								2 pedm		Slab with Guardr
								Eml	Арргоаст	STAD WILLI GUALUT
								<b> </b> Bar 7P4 ***		
LAST REVISION <b>07/01/05</b>	ISIO	RIPTION:			FDOT	2016 STANDARDS		TRAFFIC RA	AILING - (	CORRAL S



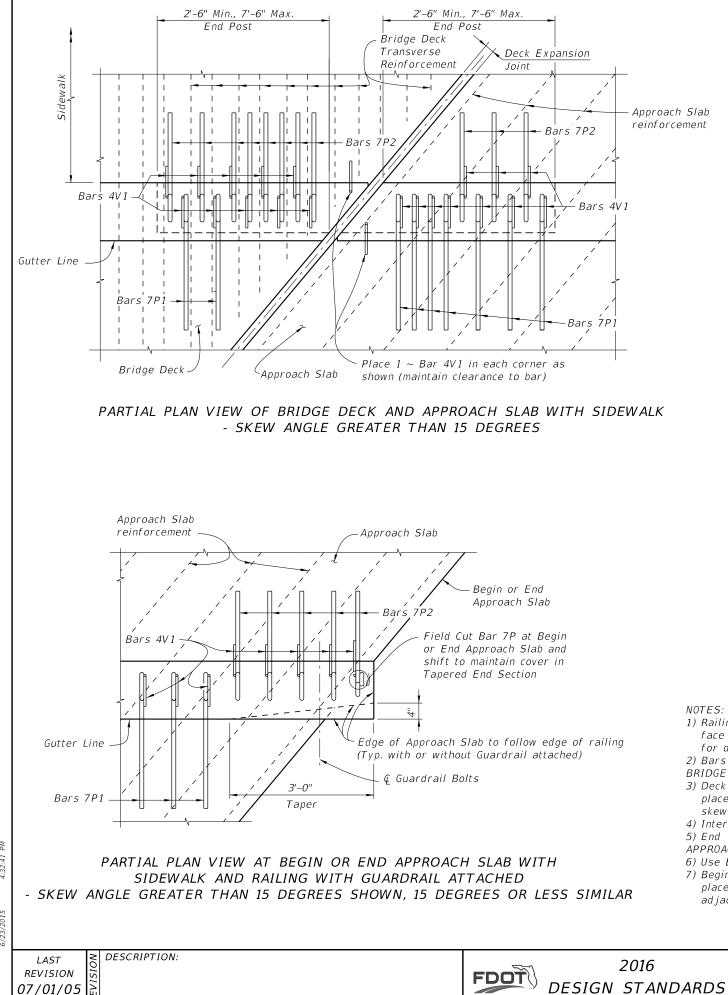


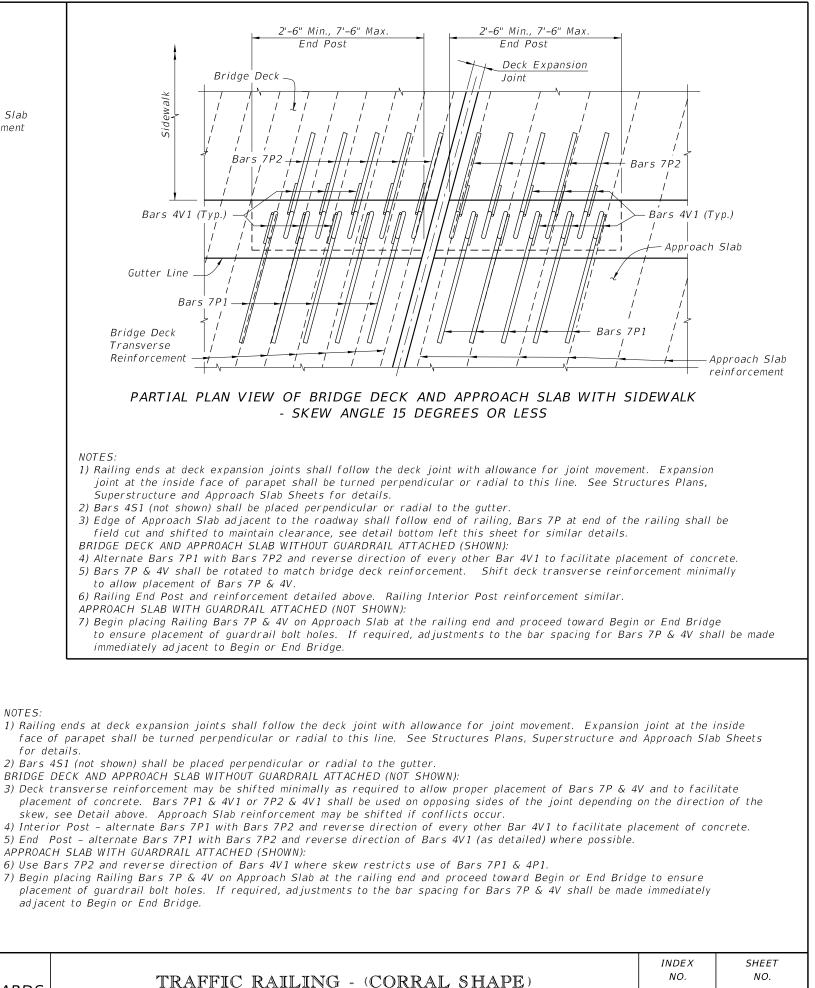




LAST REVISION

2016 FDOT DESIGN STANDARDS





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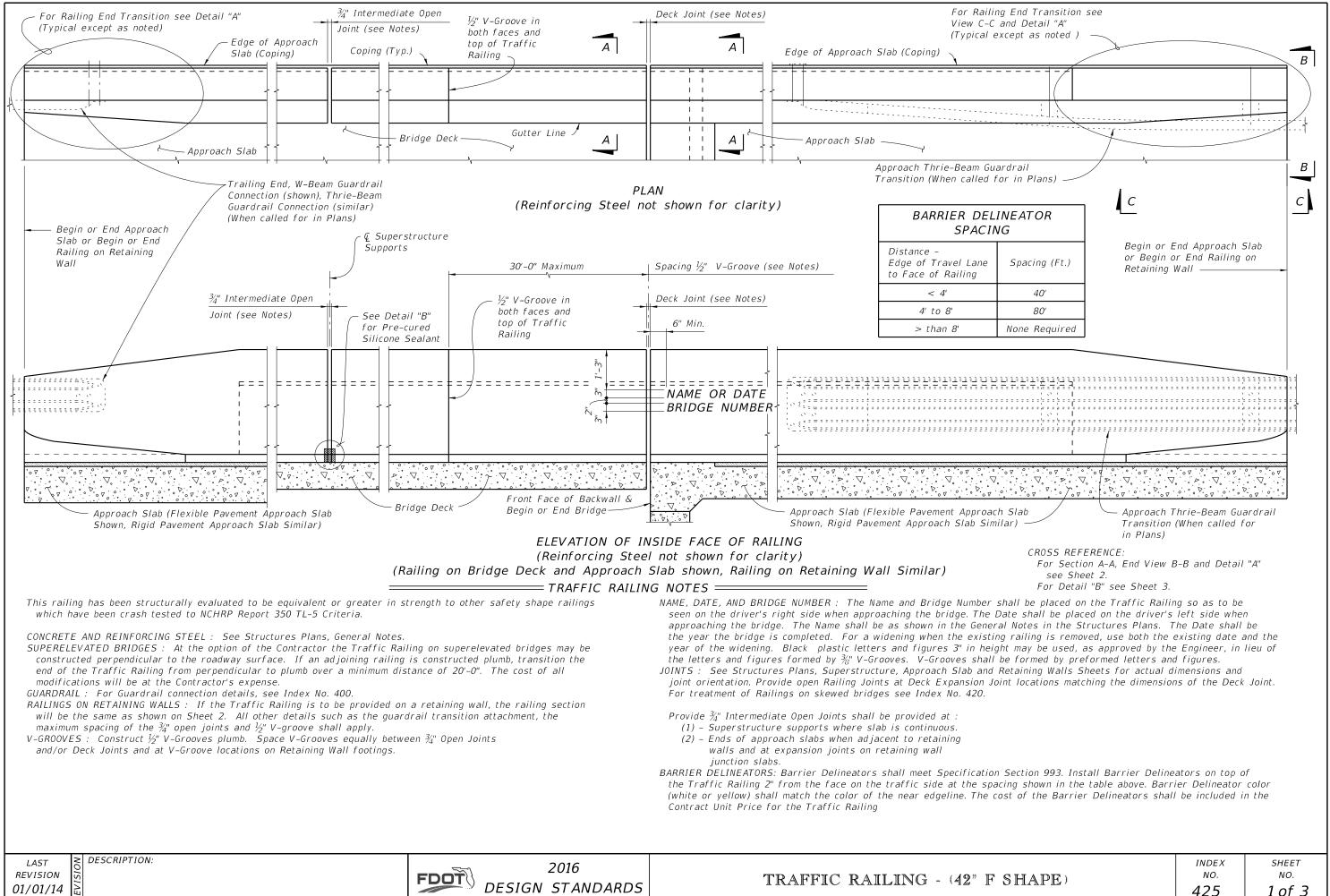
NOTES:

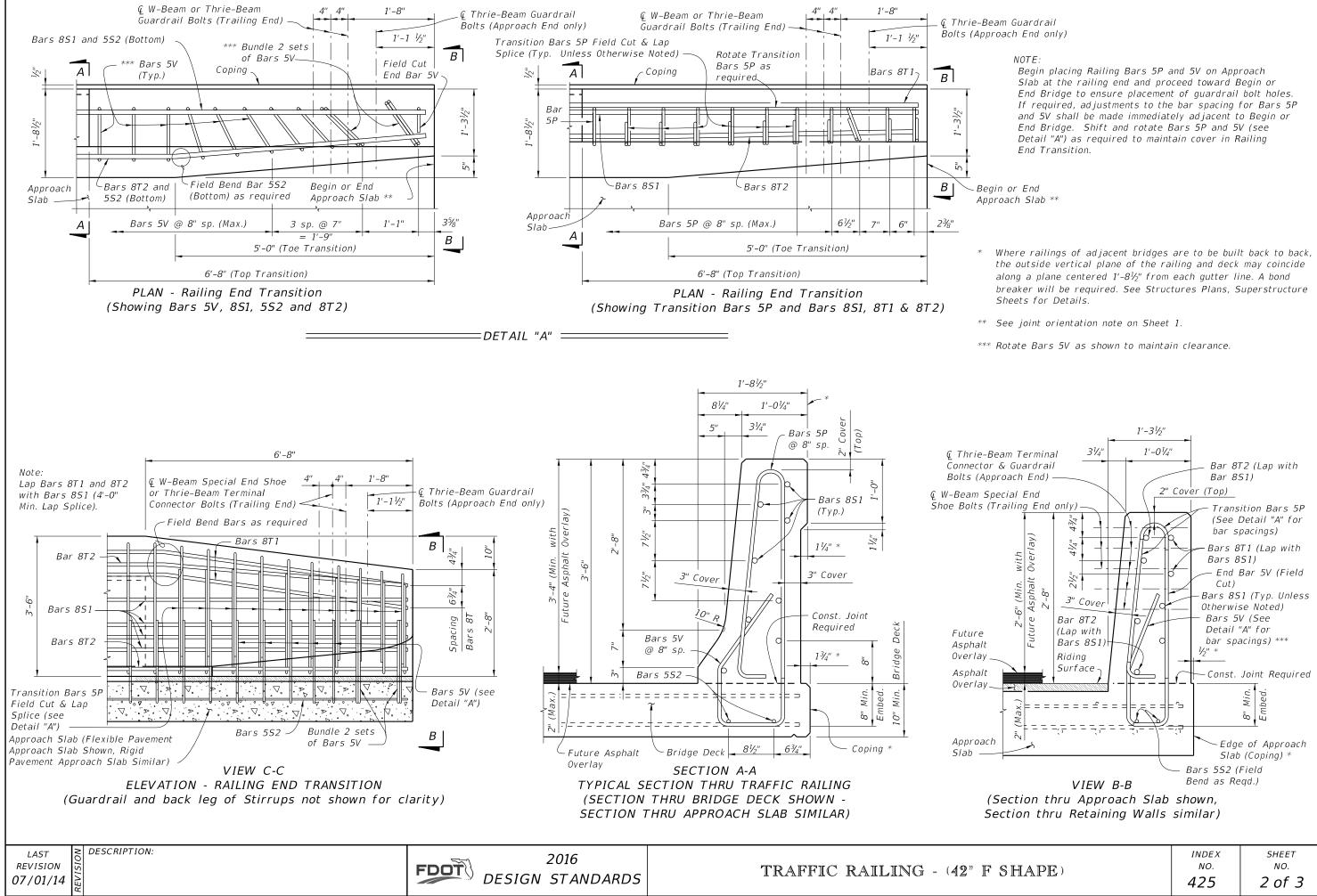
- Superstructure and Approach Slab Sheets for details.

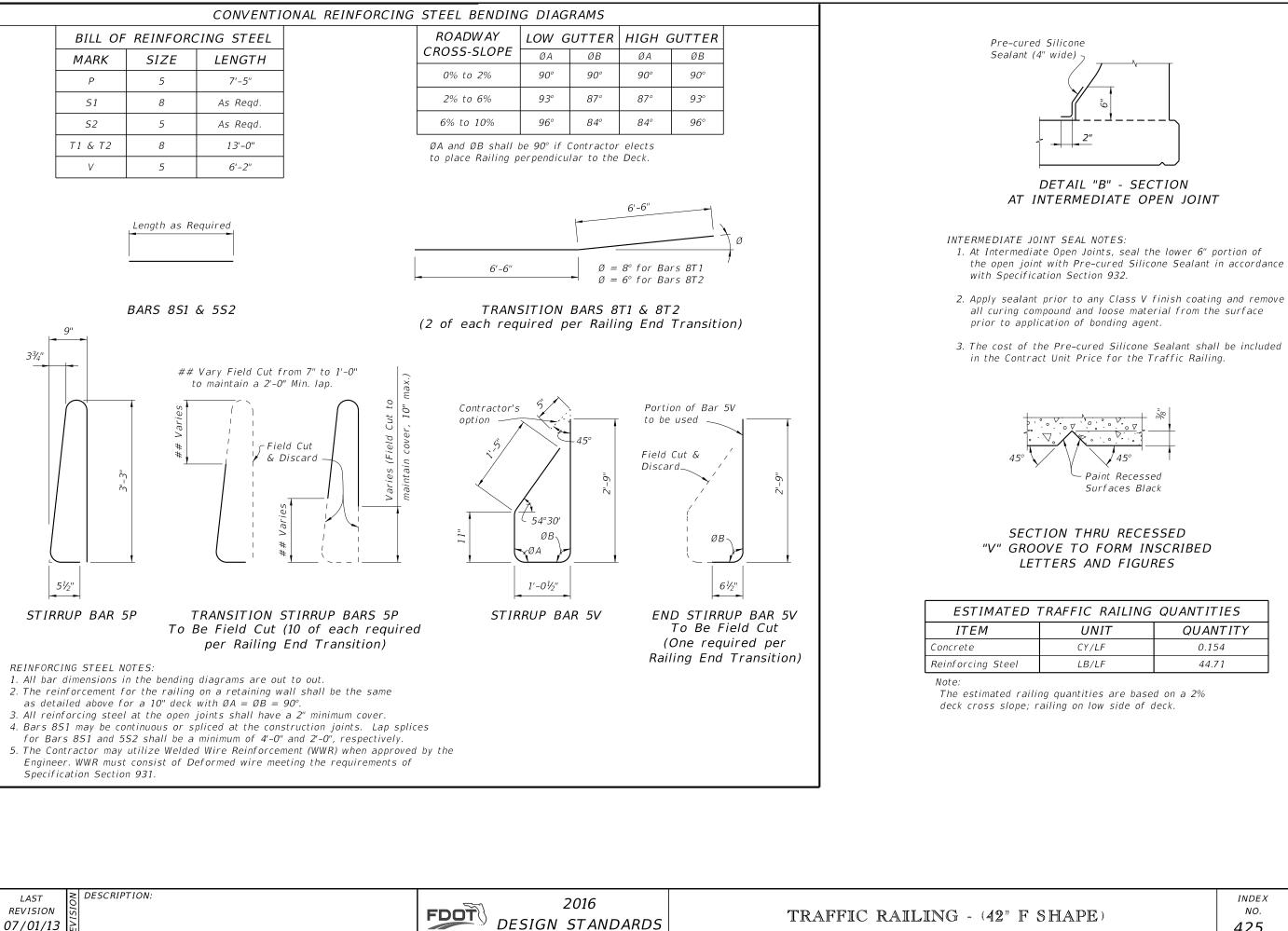
- to allow placement of Bars 7P & 4V.
- APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- immediately adjacent to Begin or End Bridge.
- for details.

2) Bars 451 (not shown) shall be placed perpendicular or radial to the gutter.

- BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (NOT SHOWN):
- APPROACH SLAB WITH GUARDRAIL ATTACHED (SHOWN):
- 6) Use Bars 7P2 and reverse direction of Bars 4V1 where skew restricts use of Bars 7P1 & 4P1.
- adjacent to Begin or End Bridge.



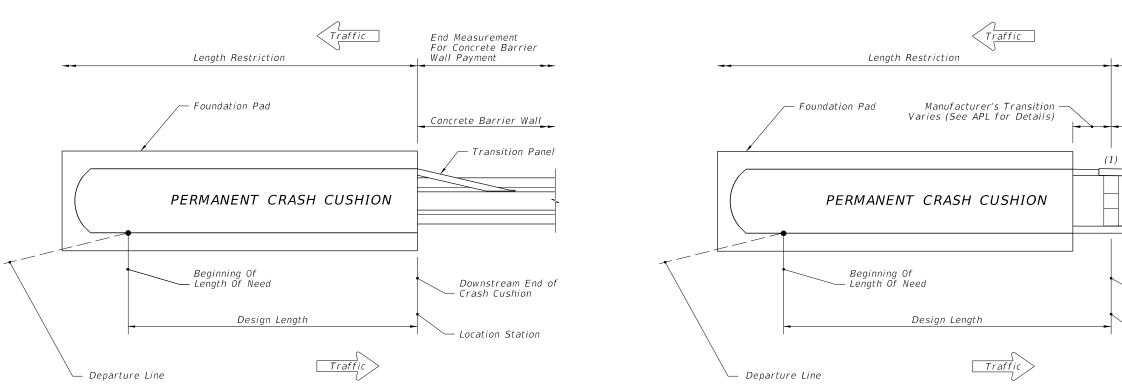




the open joint with Pre-cured Silicone Sealant in accordance

FFIC RAILING	QUANTITIES
UNIT	QUANTITY
CY/LF	0.154
LB/LF	44.71

	INDEX	SHEET
ADF)	NO.	NO.
	425	3 of 3



### GENERAL NOTES

- 1. Index 430 is applicable for permanent crash cushion installations that shield the ends of Concrete Barrier Wall or Guardrail, only.
- Design Length is based on a given design speed and the shortest Crash Cushion available on the 2. Approved Products List (APL). When a Length Restriction is not applicable (N/A), then the Contractor has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans. When a Length Restriction is applicable, then the Contractor has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans and that are less than or equal to the Length Restriction identified in the plans.
- For High Speed Facilities with a Design Speed greater than 60 mph, use a TL-3 Crash Cushion. З.
- Assemble and install Crash Cushions according to the limitations noted on the Approved Products 4. List (APL) webpage, the manufacturer's specifications, and the applicable crash cushion drawings posted on the APL.
- When subjected to reverse direction hits, construct Transition Panels from Concrete Barrier Walls 5. to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.
- Galvanize metallic components to meet the requirements for Steel Guardrail, Section 967 of the Standard Specifications for Road and Bridge Construction. 6.
- For Guardrail Applications, construct the Manufacturer's Transition between the Permanent Crash 7. Cushion and the Standard Guardrail Transition; refer to all Standard Guardrail Transition details of this index.
- 8. For additional information on the End Measurement for Guardrail Payment, refer to the Standard Specifications for Road and Bridge Construction, Section 536.
- 9 A yellow Type I Object Marker shall be centered 3' in front of the crash cushion nose. As an option, the contractor may install Retroreflective Sheeting on the nose of the crash cushion. The sheeting to be used must be solid yellow, Type IV or better and must be a product listed on the Department's Approved Products List (APL). The sheeting to be applied to the noise of the crash cushion shall be a minimum of 360 square inches with a minimum height of 15 inches. Mounting hardware, Object Markers or Retroreflective Sheeting shall be in conformance with Section 993 of the Standard Specifications for Road and Bridge Construction.
- 10. The EOR shall provide the station of the Length of Need (LON) location in the plans.

G
Design Leng (ft.)
8.75
11.50
11.50
14.25
20.00
22.75

DESCRIPTION:

Concrete Barrier Wall Applications

Design Speed

(mph)

35

40

45

50

55

≥ 60

Design Length

(ft.)

5.75

7.25

7.25

10.25

13.25

16.00

Crash

Test Level

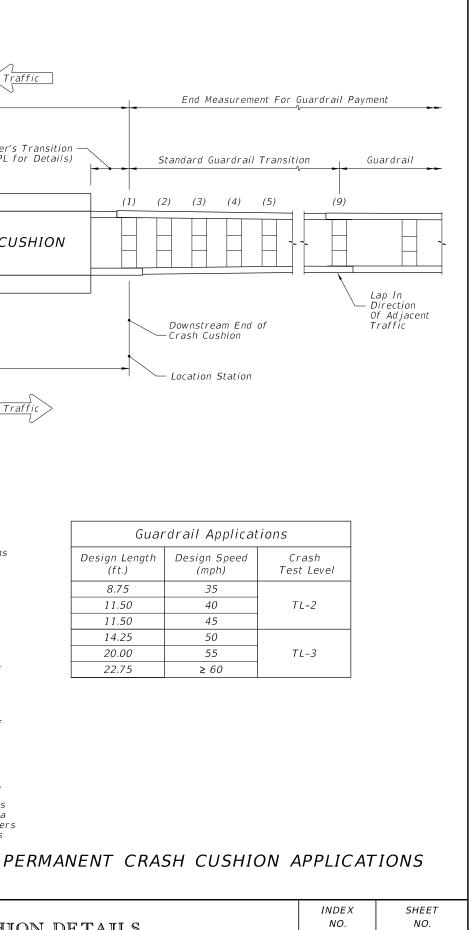
TL-2

TL-3



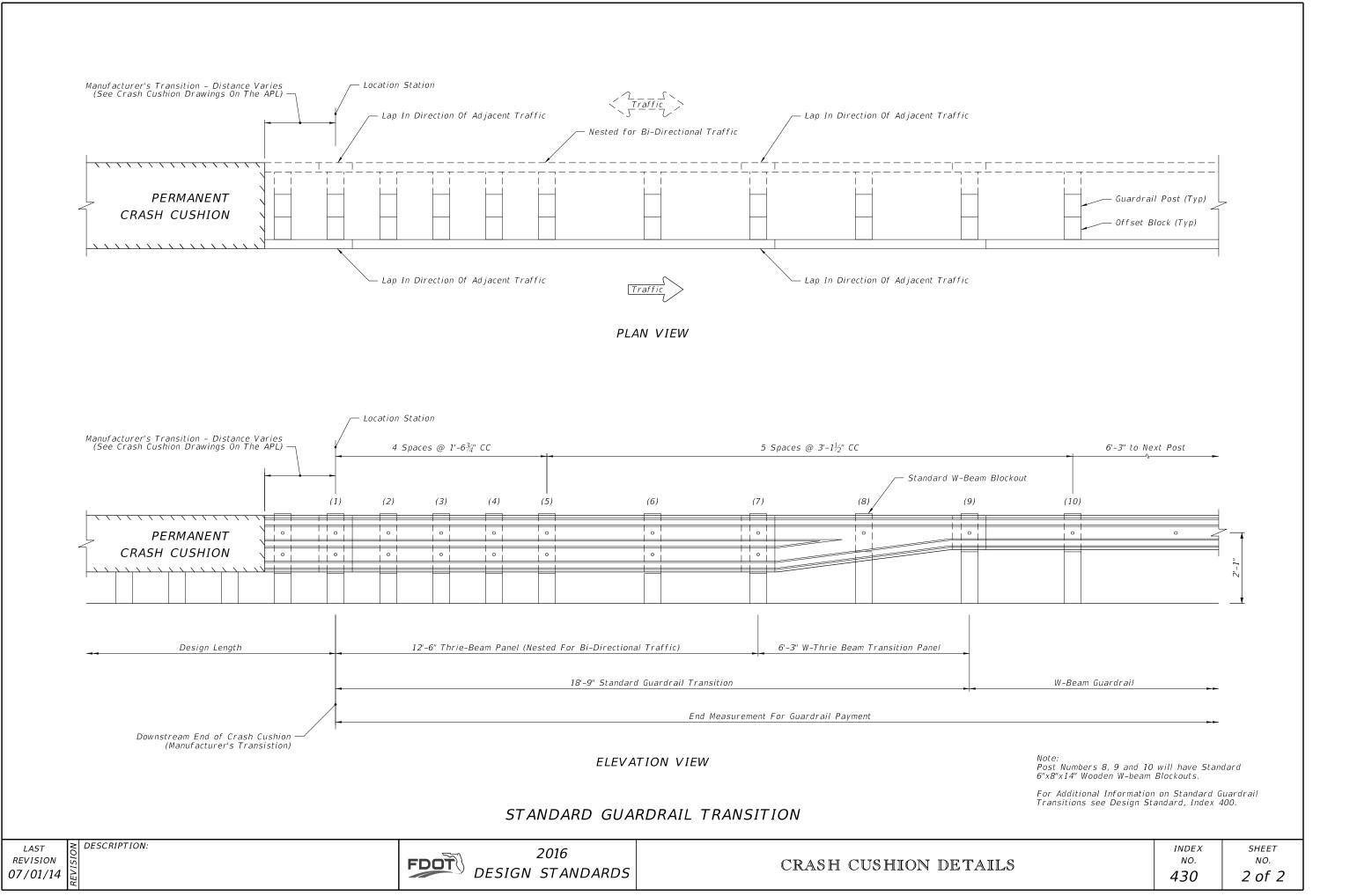
2016 DESIGN STANDARDS

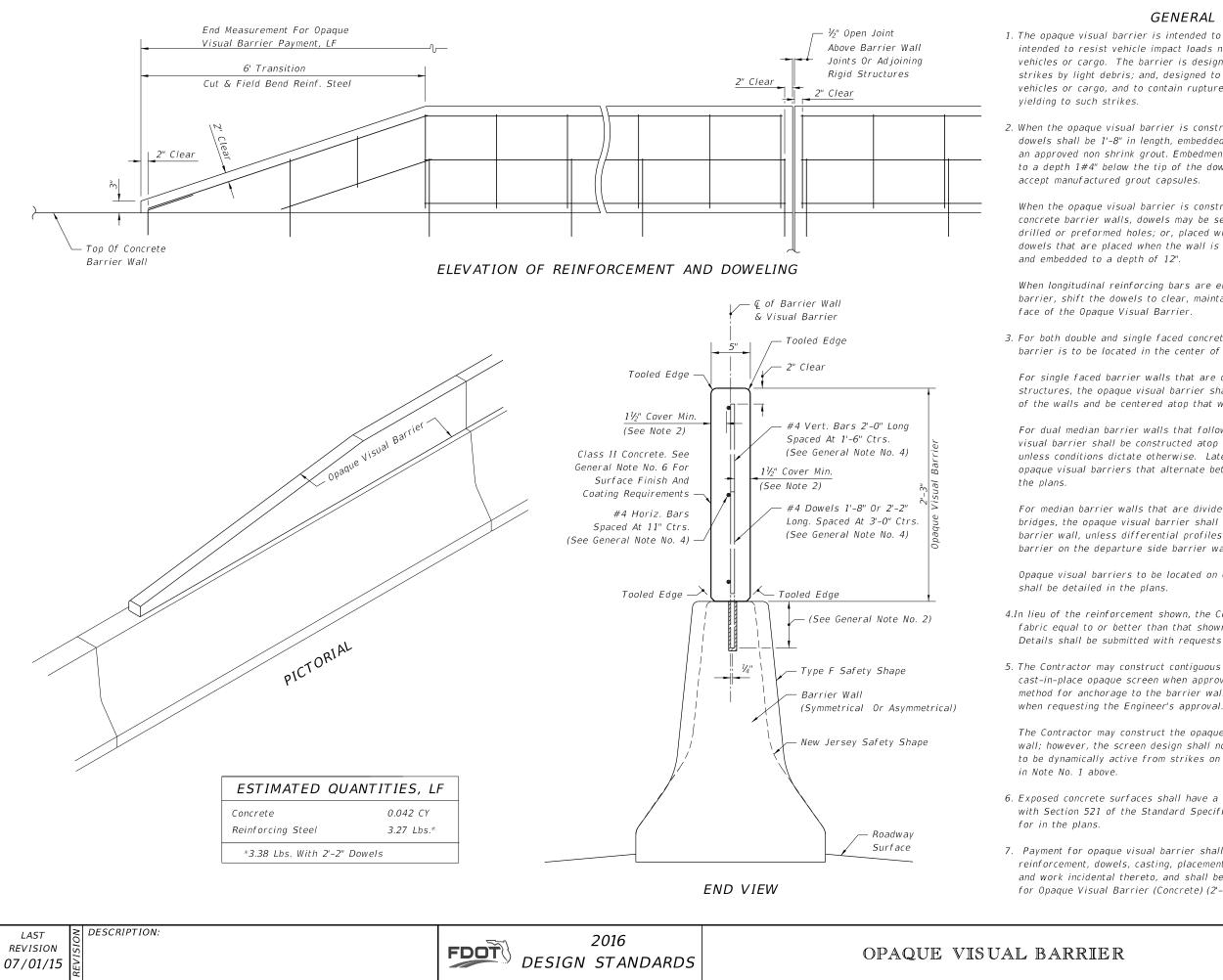
CRASH CUSHION DETAILS



430

1 of 2





# GENERAL NOTES

1. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when

2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved non shrink grout. Embedment holes shall be 5#8" diameter, drilled to a depth 1#4" below the tip of the dowel unless greater depth is required to

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the 11#2" Cover Minimum to the

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls

4.In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer. Details shall be submitted with requests for substitution.

5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations

6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called

7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.

	INDEX	SHEET
₹	NO.	NO.
I.	461	1 of 1

### \_\_\_\_\_ TRAFFIC RAILING NOTES \_\_\_\_\_

This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be  $\frac{3}{4}$ " by 2<sup>1</sup>/<sub>2</sub>" slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

- GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.
- ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM A449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.
- COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.
- ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for  $\frac{7}{3}$ " Ø anchor bolts; 55,000 lbs. for the  $1\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the  $1\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3'' or  $3'-1\frac{1}{2}''$  spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of quardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

- THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten quardrail bolts in  $3\frac{3}{4}$ " slots at quardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.
- NEOPRENE PADS: Neoprene pads must be plain pads with a durometer hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required.
- ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.
- BARRIER DELINEATORS: Barrier Delineators shall conform to Spec. Section 993. Install Barrier Delineators at the top of the guardrail offset blocks at the spacings shown in the table below. Barrier Delineator color (white or yellow) shall conform to the color of the near edgeline.
- PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index No. 400 for details.
- BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.
- PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

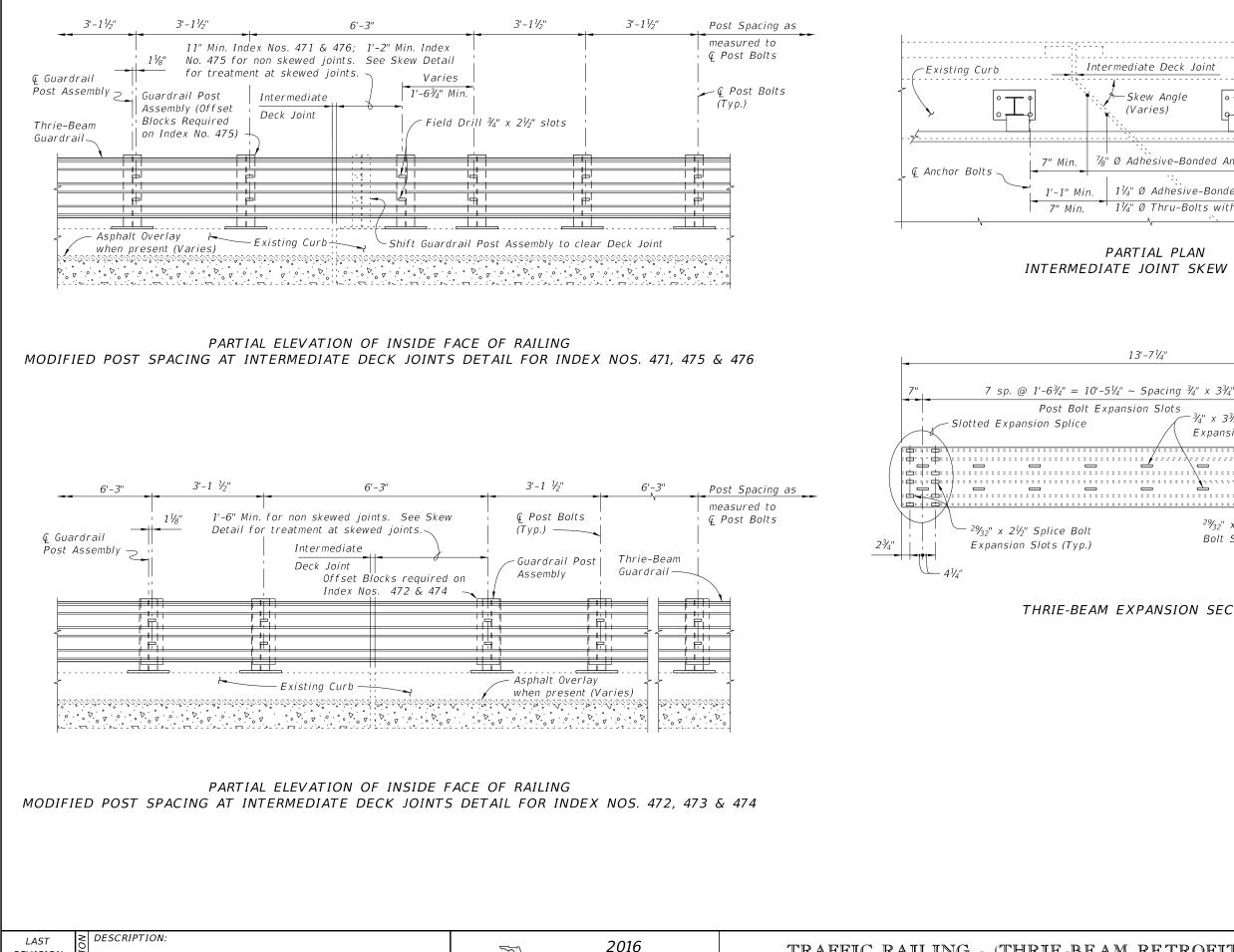
BARRIER DELII SPACINO	
Distance – Edge of Travel Lane to Face of Railing	Spac
< 4'	
4' to 8'	
> than 8'	None

<	IDESCRIPTION
5	
1	
S	
-	
$\geq$	



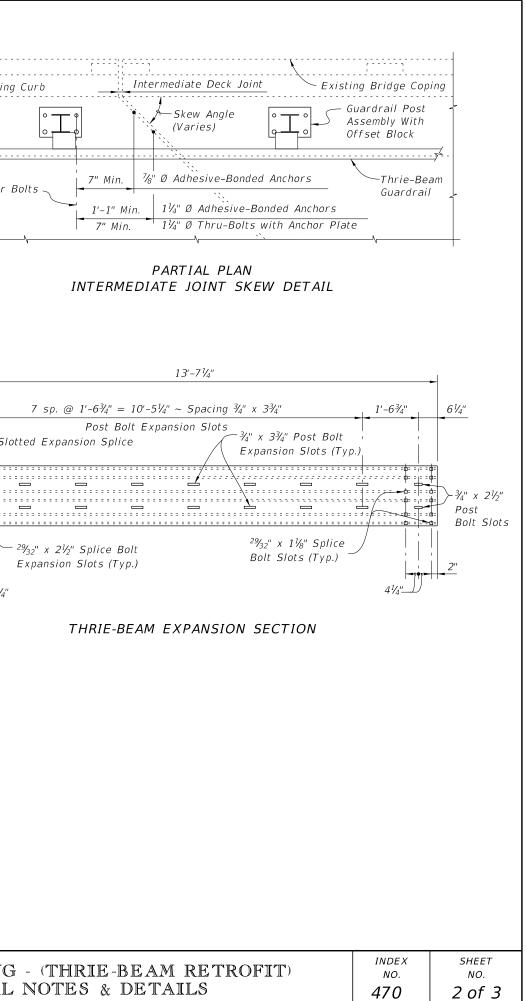
OR
cing (Ft.)
40'
80'
Required

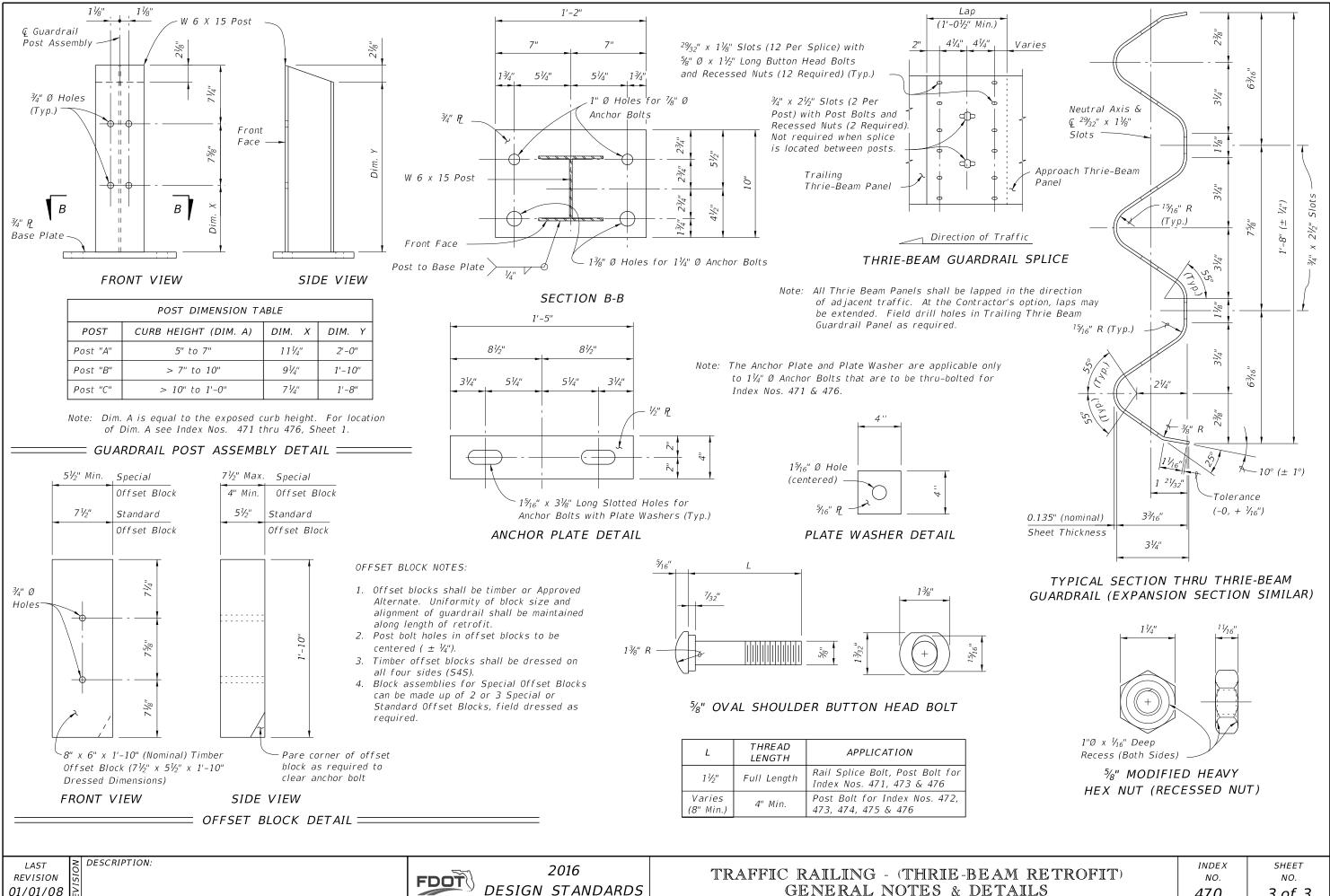
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FDOT DESIGN STANDARDS TRAFFIC RAILING - (THRIE-BEAM RETROFIT) GENERAL NOTES & DETAILS



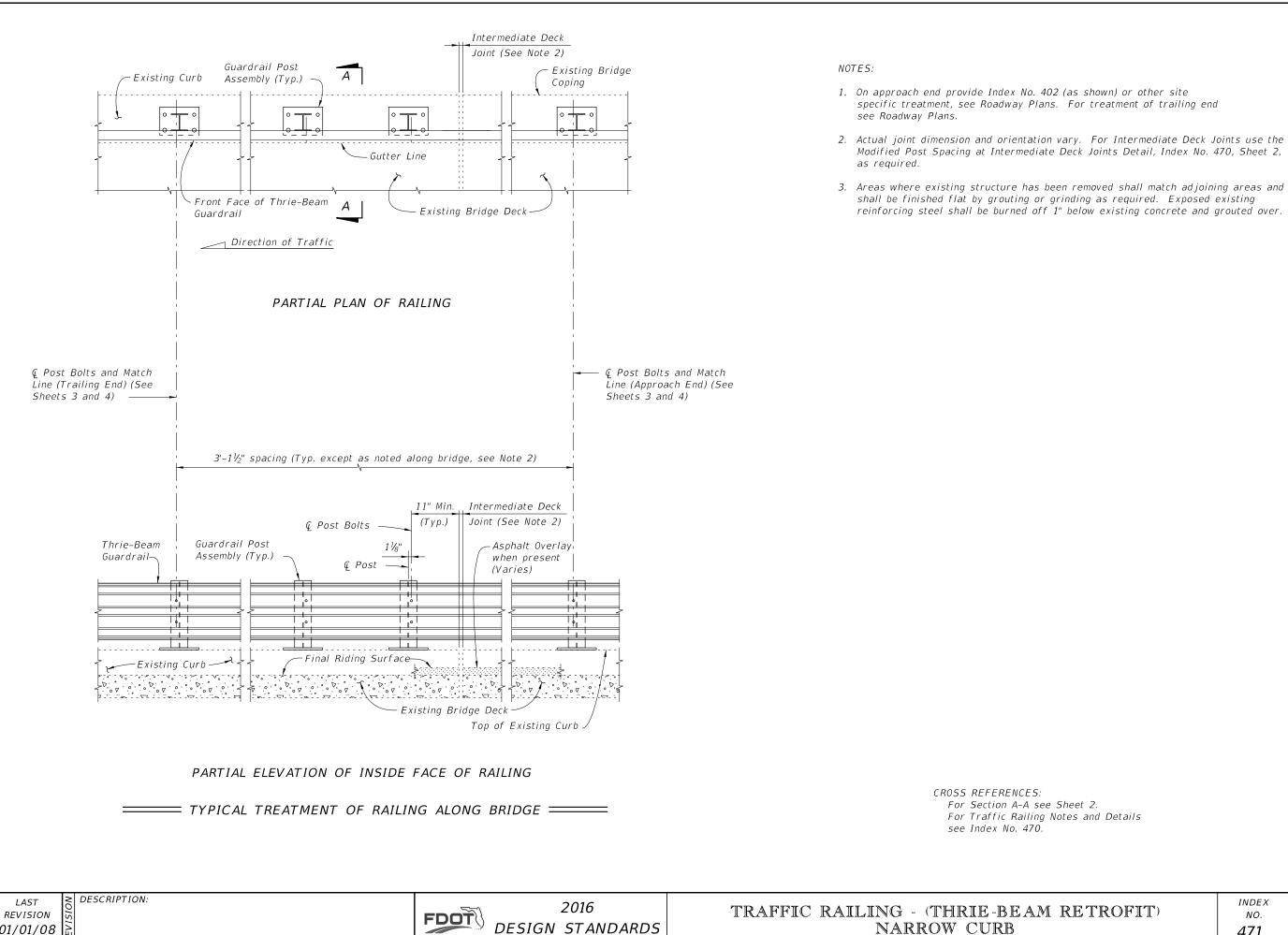


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GENERAL NOTES & DETAILS

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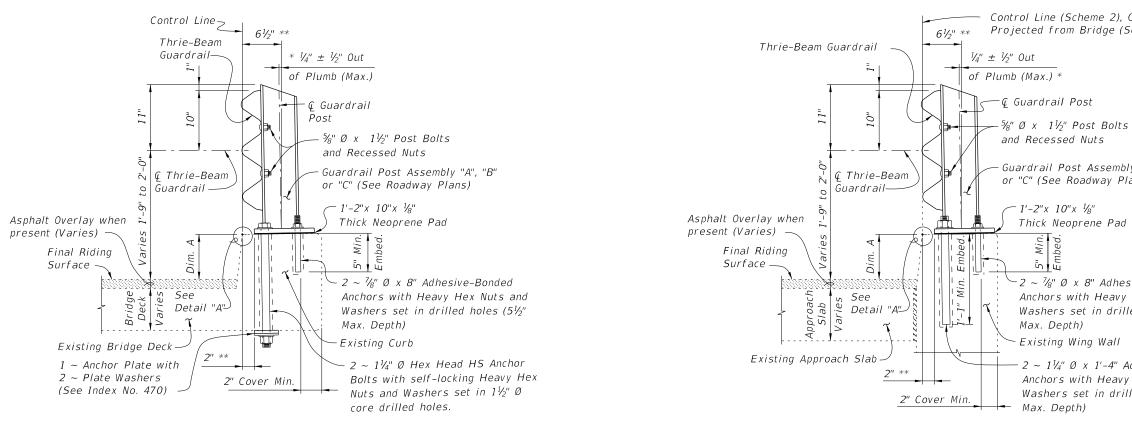


NARROW CURB

Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2,

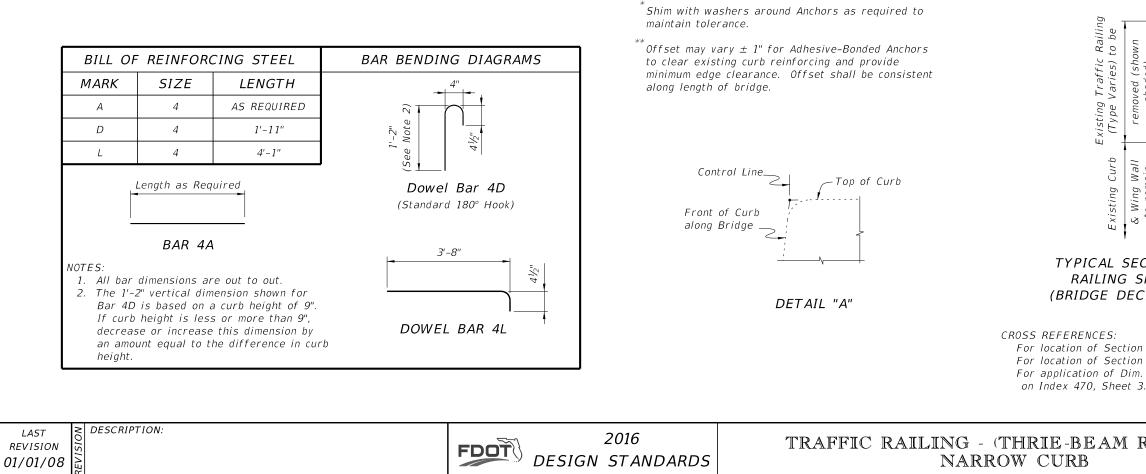
reinforcing steel shall be burned off 1" below existing concrete and grouted over.

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SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)



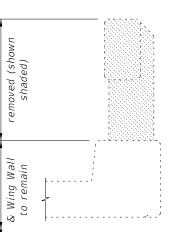
Control Line (Scheme 2), Control Line Projected from Bridge (Scheme 3)

Guardrail Post Assembly "A", "B" or "C" (See Roadway Plans)

Thick Neoprene Pad

 $-2 \sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors with Heavy Hex Nuts and Washers set in drilled holes  $(5\frac{1}{2})$ 

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" Adhesive-Bonded Anchors with Heavy Hex Nuts and Washers set in drilled holes  $(1'-1)^{1/2''}$ 



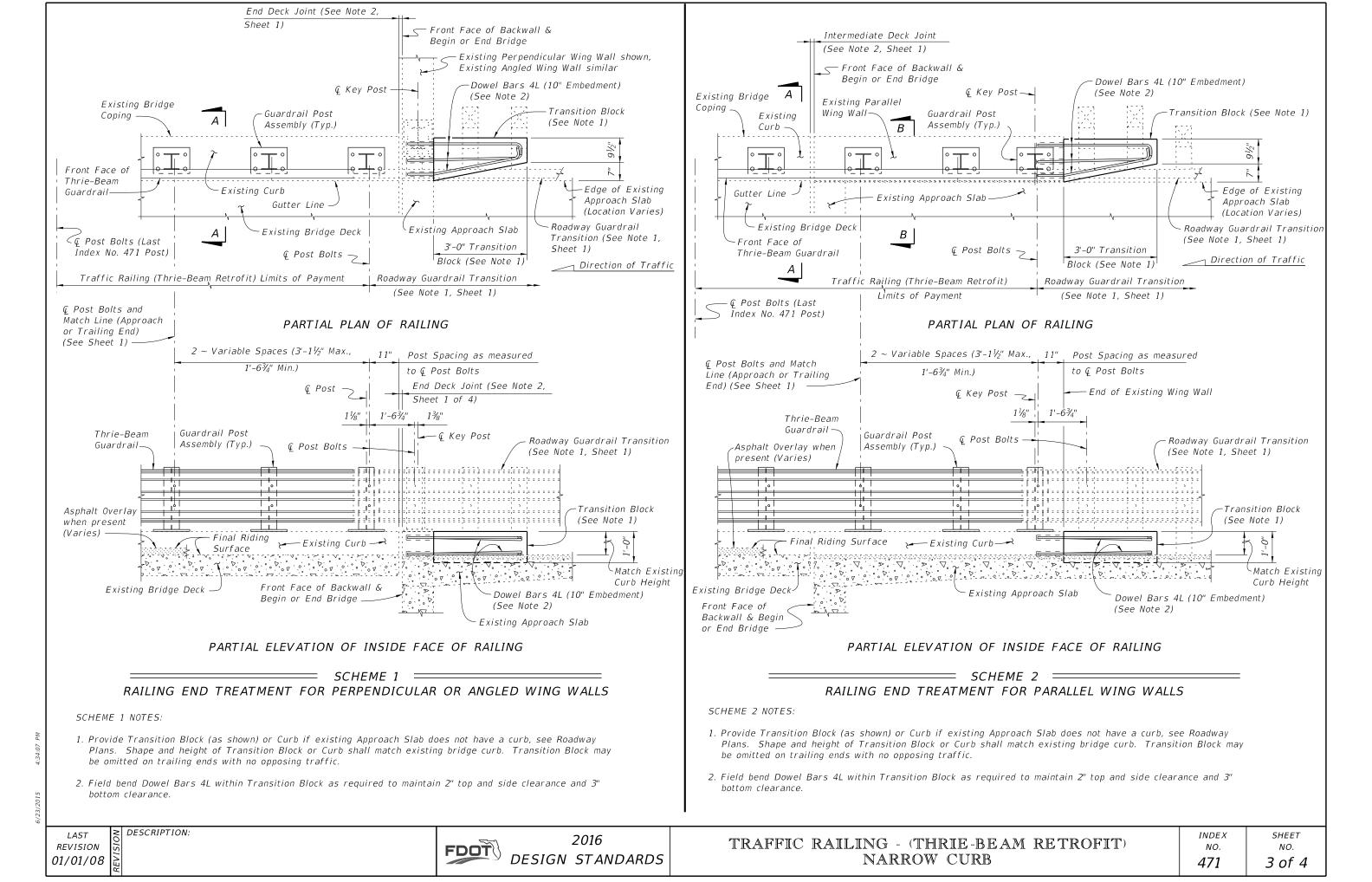
Existing Traffic Railing (Type Varies) to be

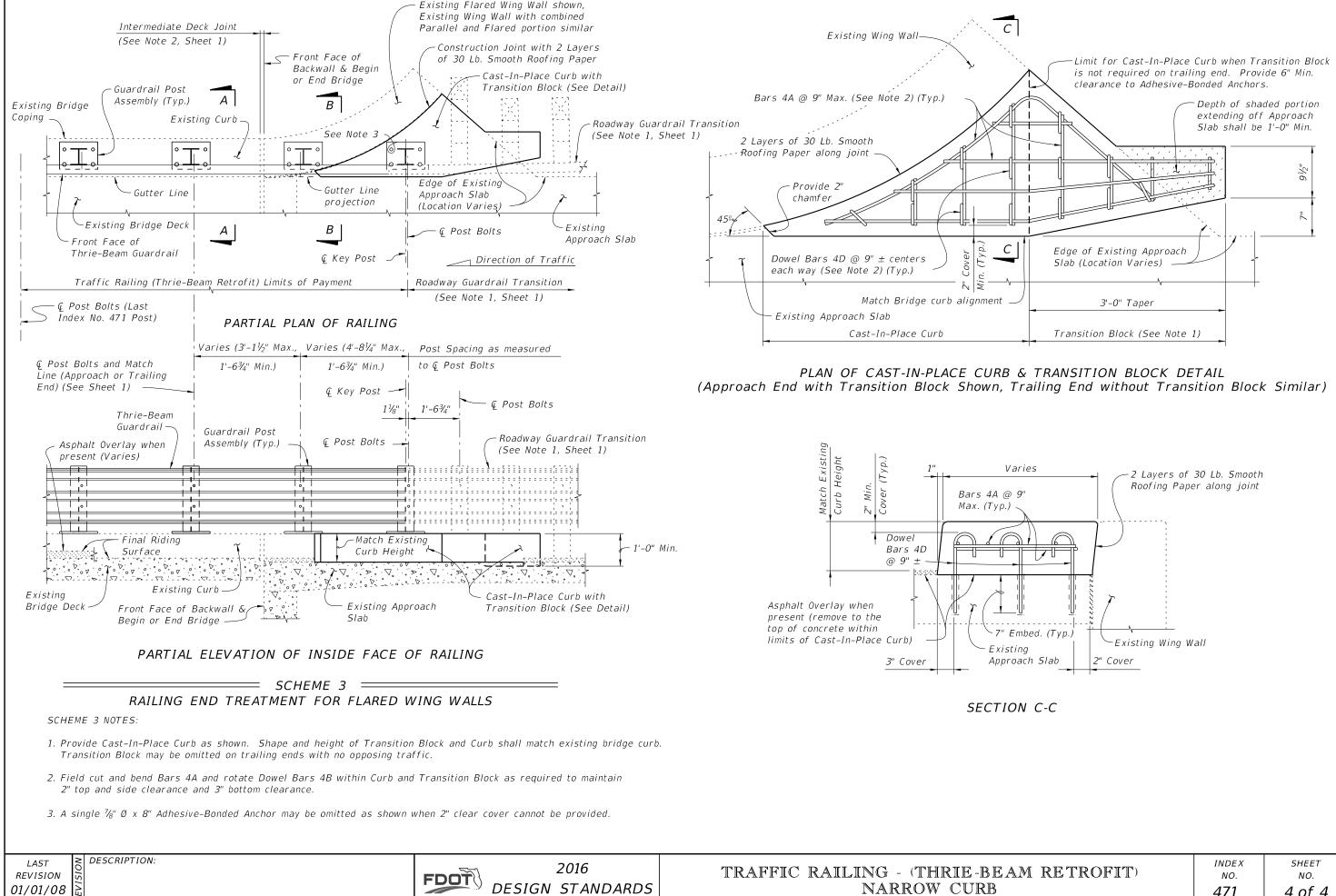
Existing Curb

# TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

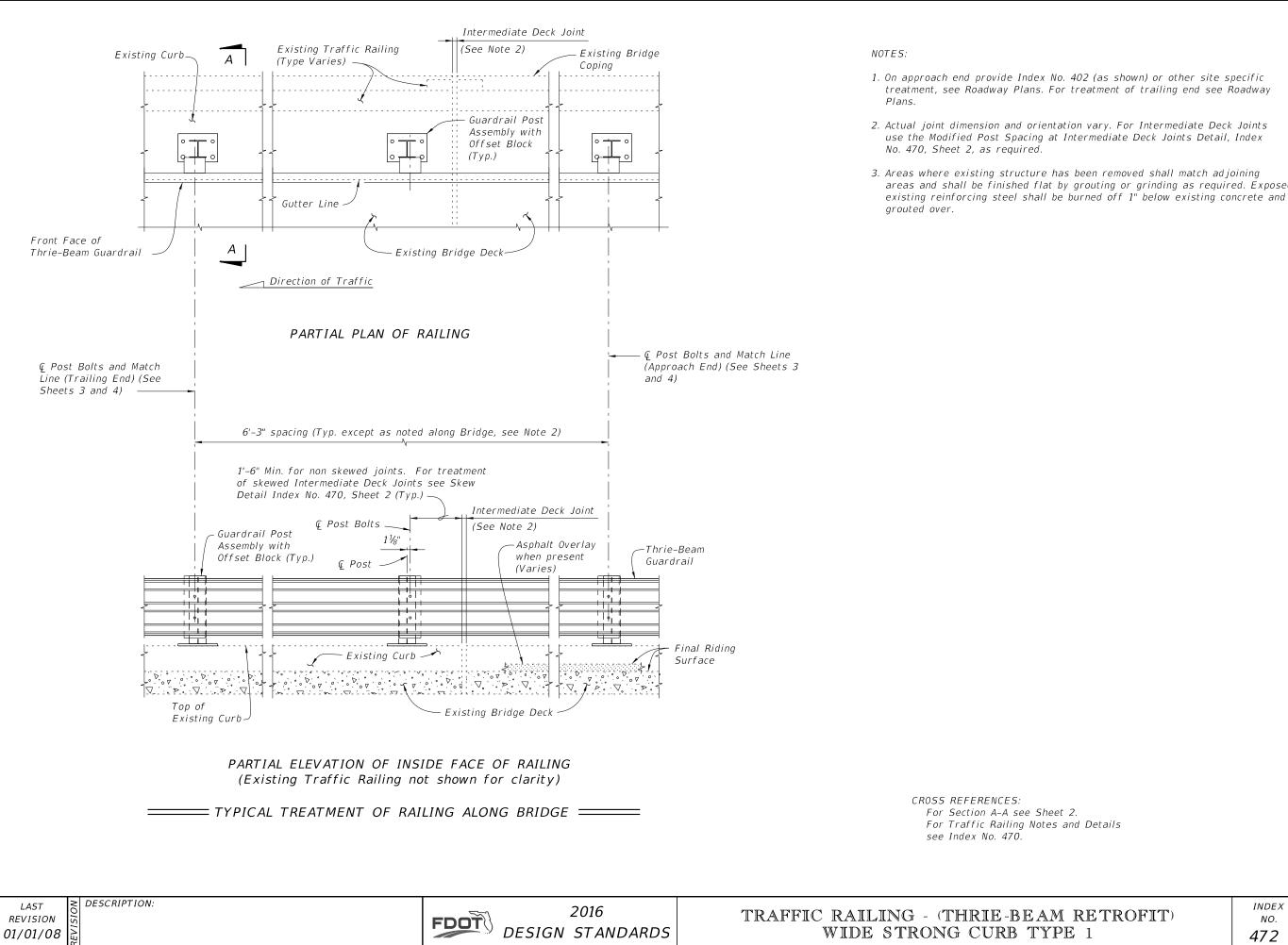
For location of Section A-A see Sheets 1, 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table

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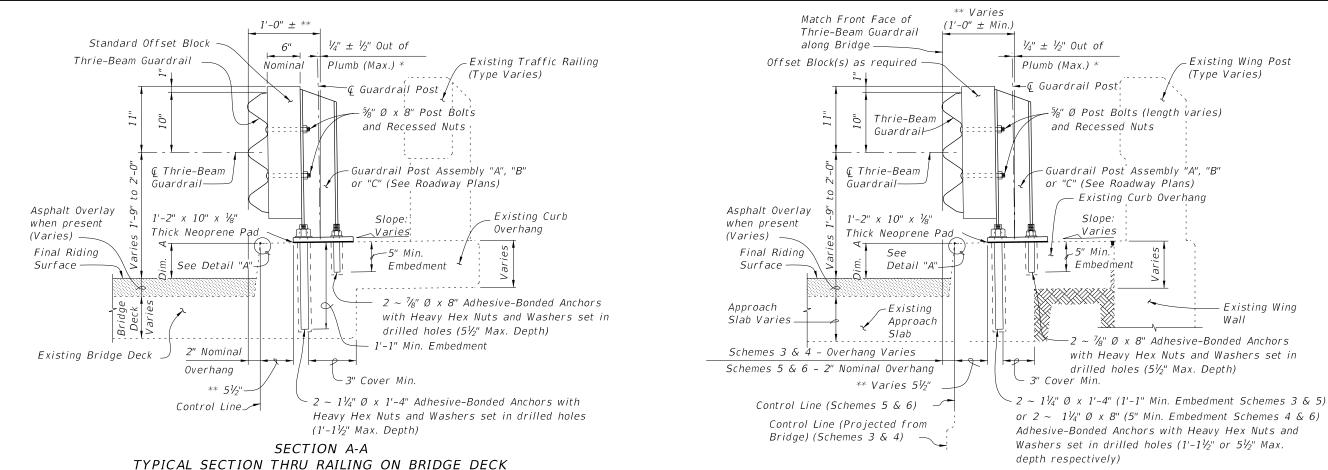


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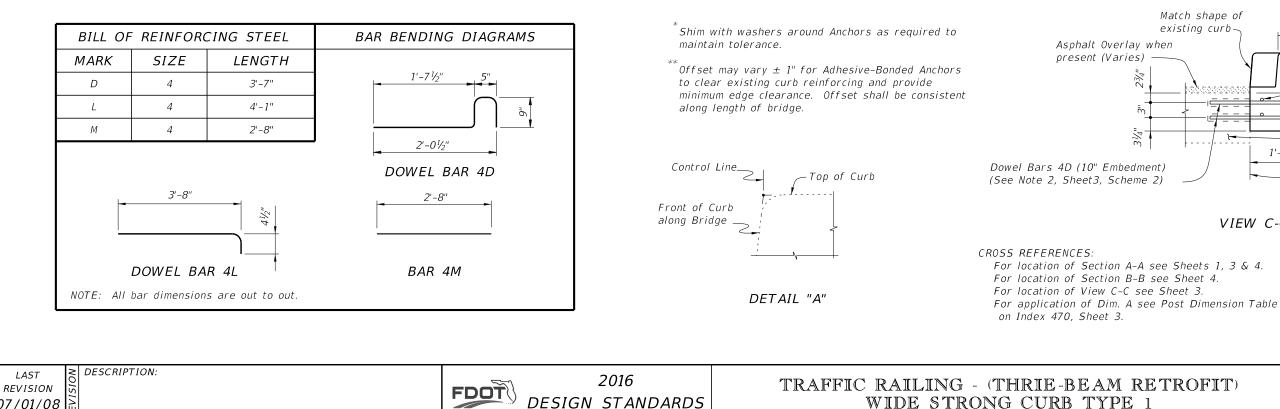


areas and shall be finished flat by grouting or grinding as required. Exposed

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SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)



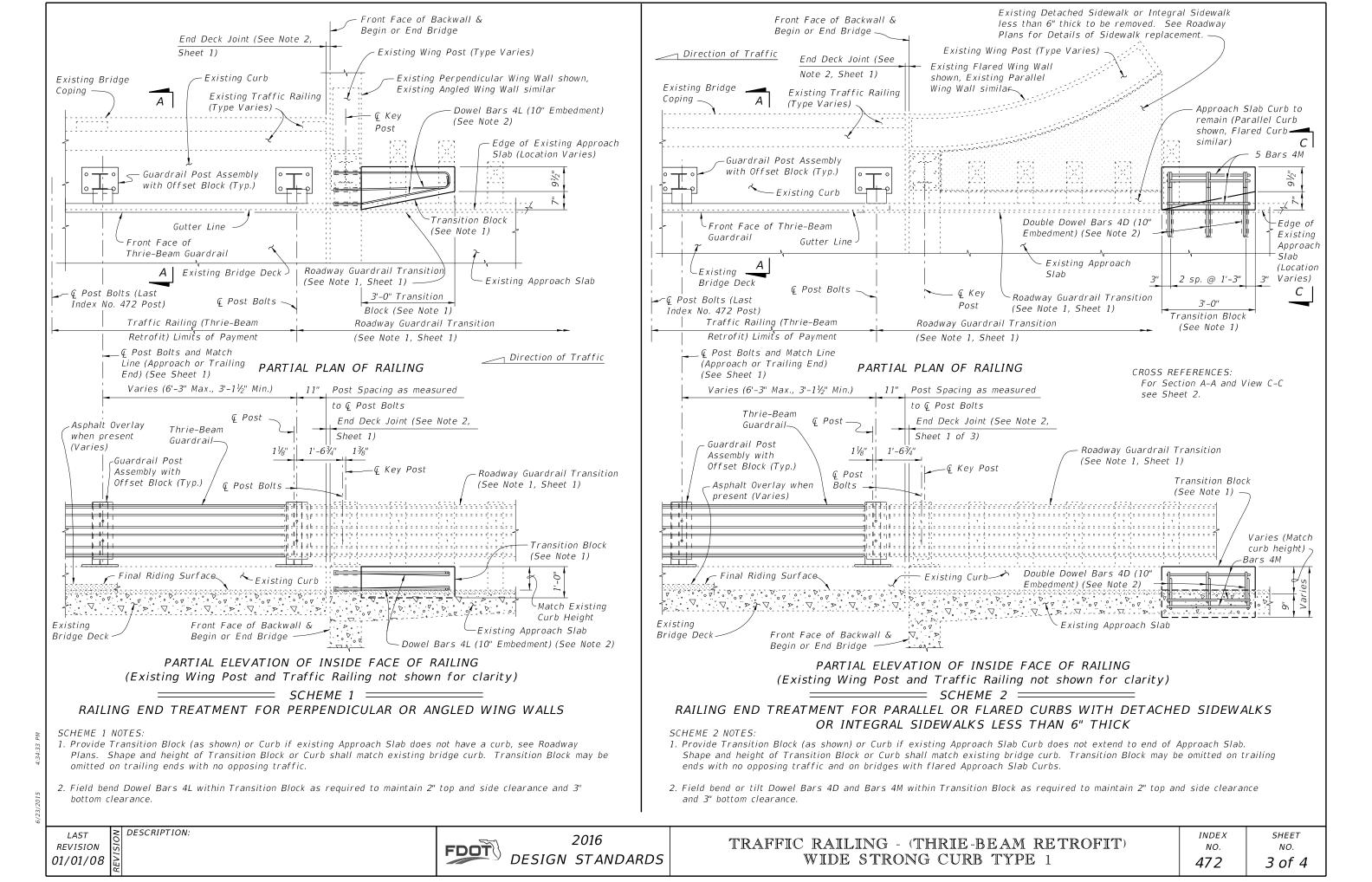
DESIGN STANDARDS

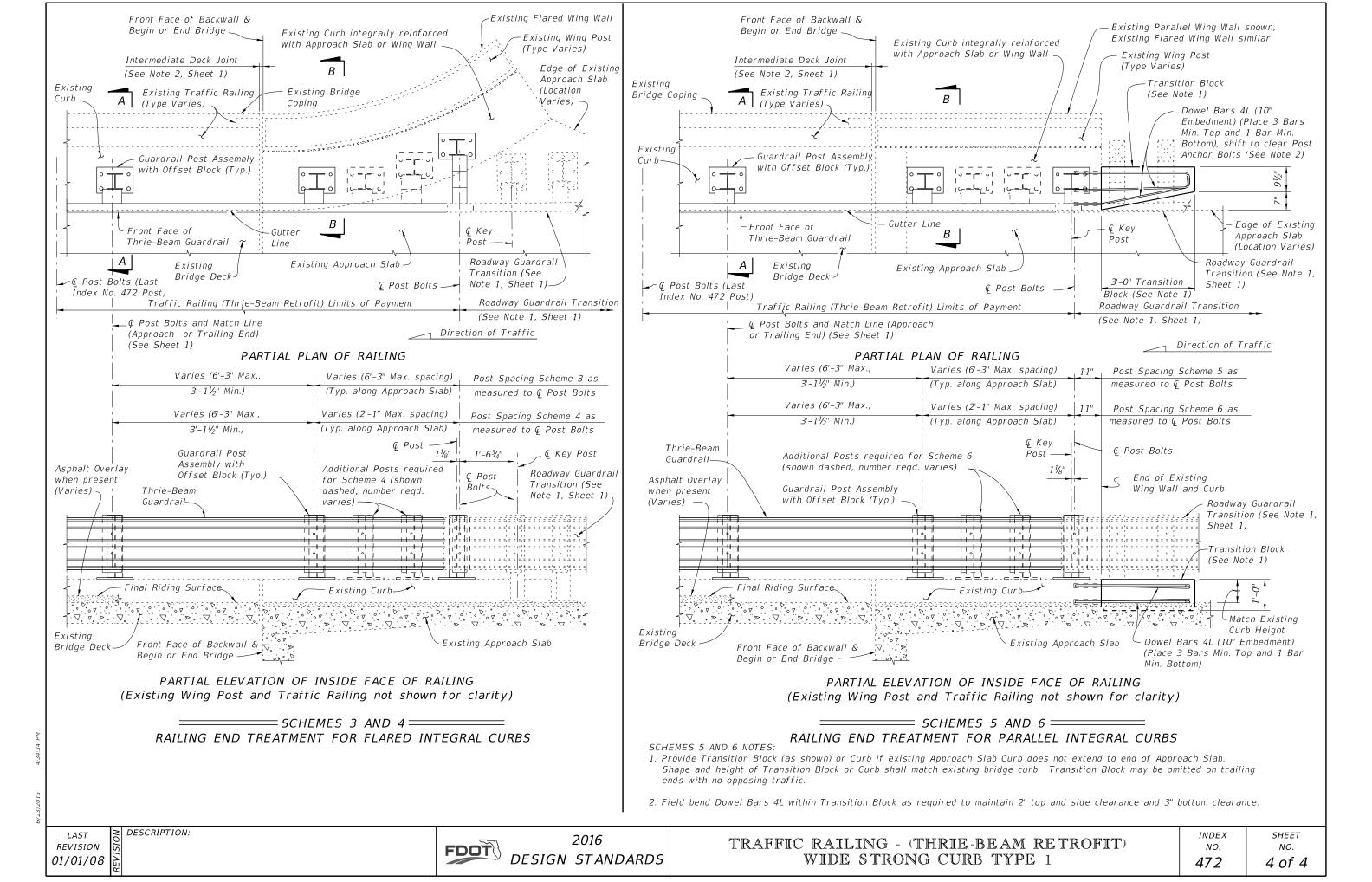
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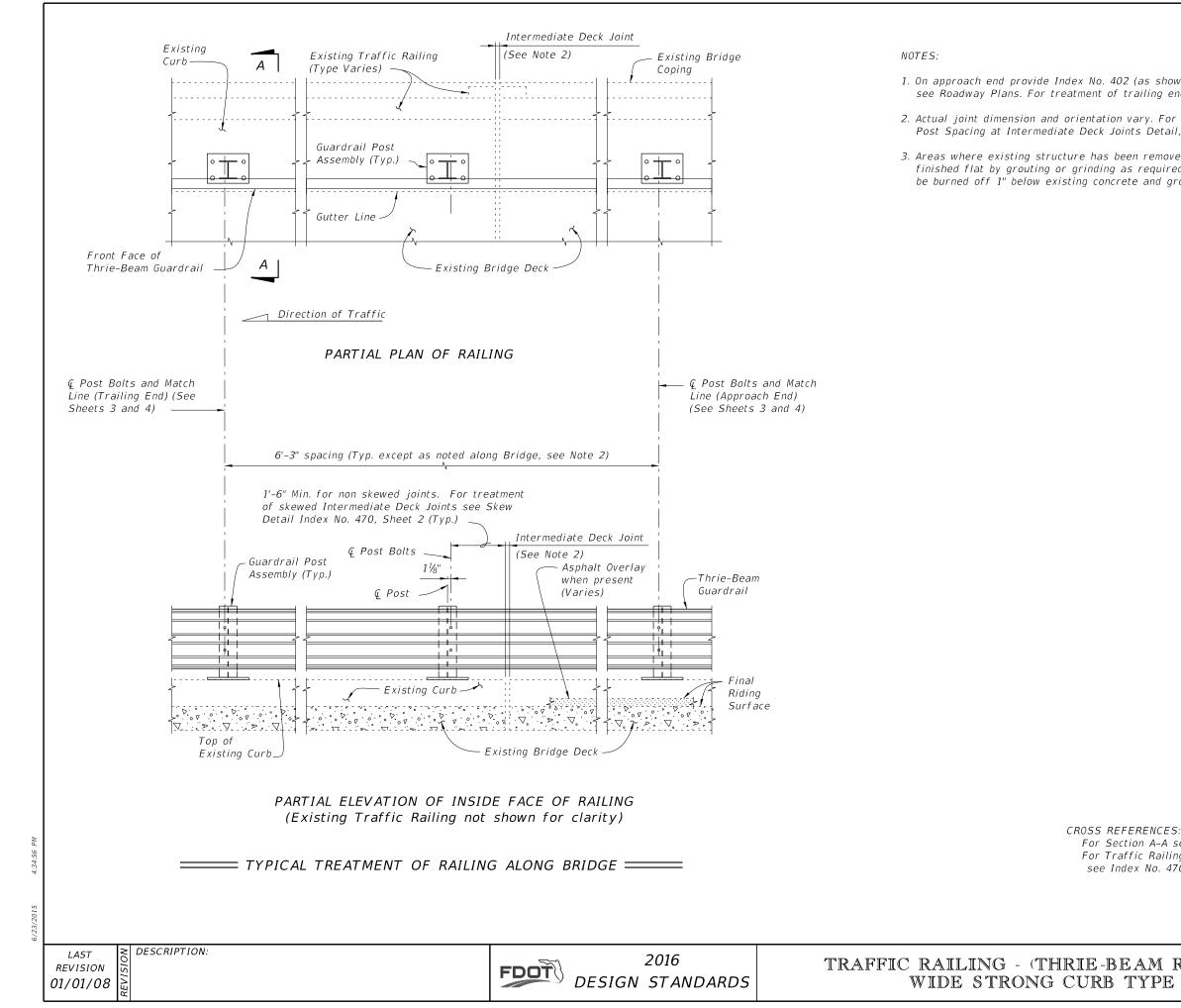
# Match shape of Varies (Match 9½" existing curb curb height) Bars 4MExisting $1' - 4\frac{1}{2}''$ Approach Slab Edge of Existing Approach Slab VIEW C-C INDEX SHEET NO. NO.

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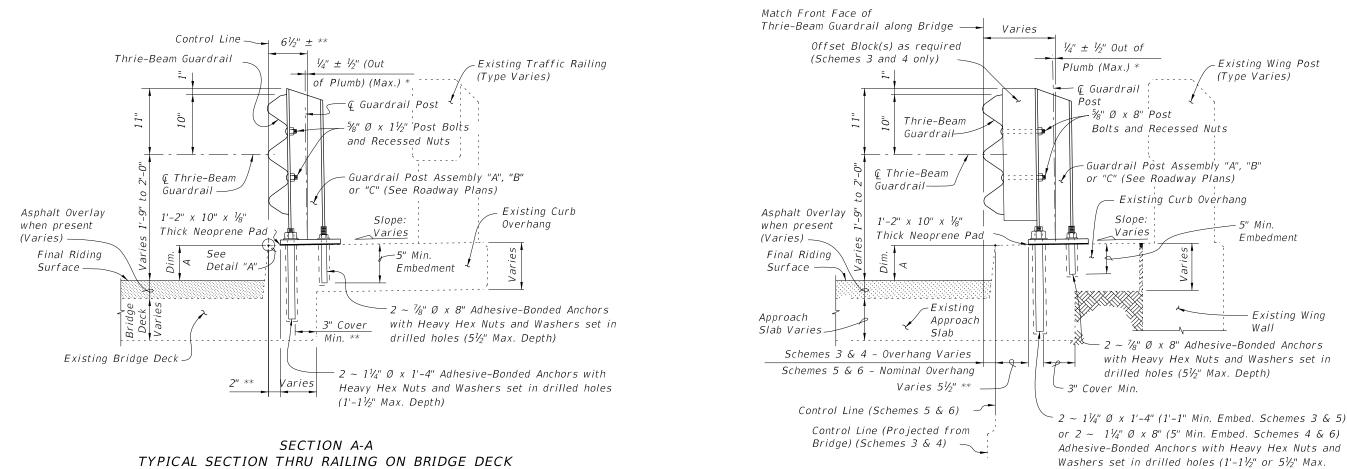
### NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

WIDE STRONG CURB TYPE

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SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

\* Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

DETAIL "A"

Control Line —>

Front of Curb along Bridge

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\*\* Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.

- Top of Curb

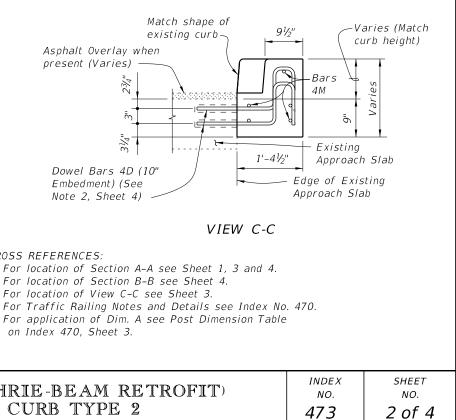
- present (Varies) 31/4
- Dowel Bars 4D (10" Embedment) (See Note 2, Sheet 4)
- CROSS REFERENCES: For location of Section B-B see Sheet 4. For location of View C-C see Sheet 3.
  - on Index 470, Sheet 3.

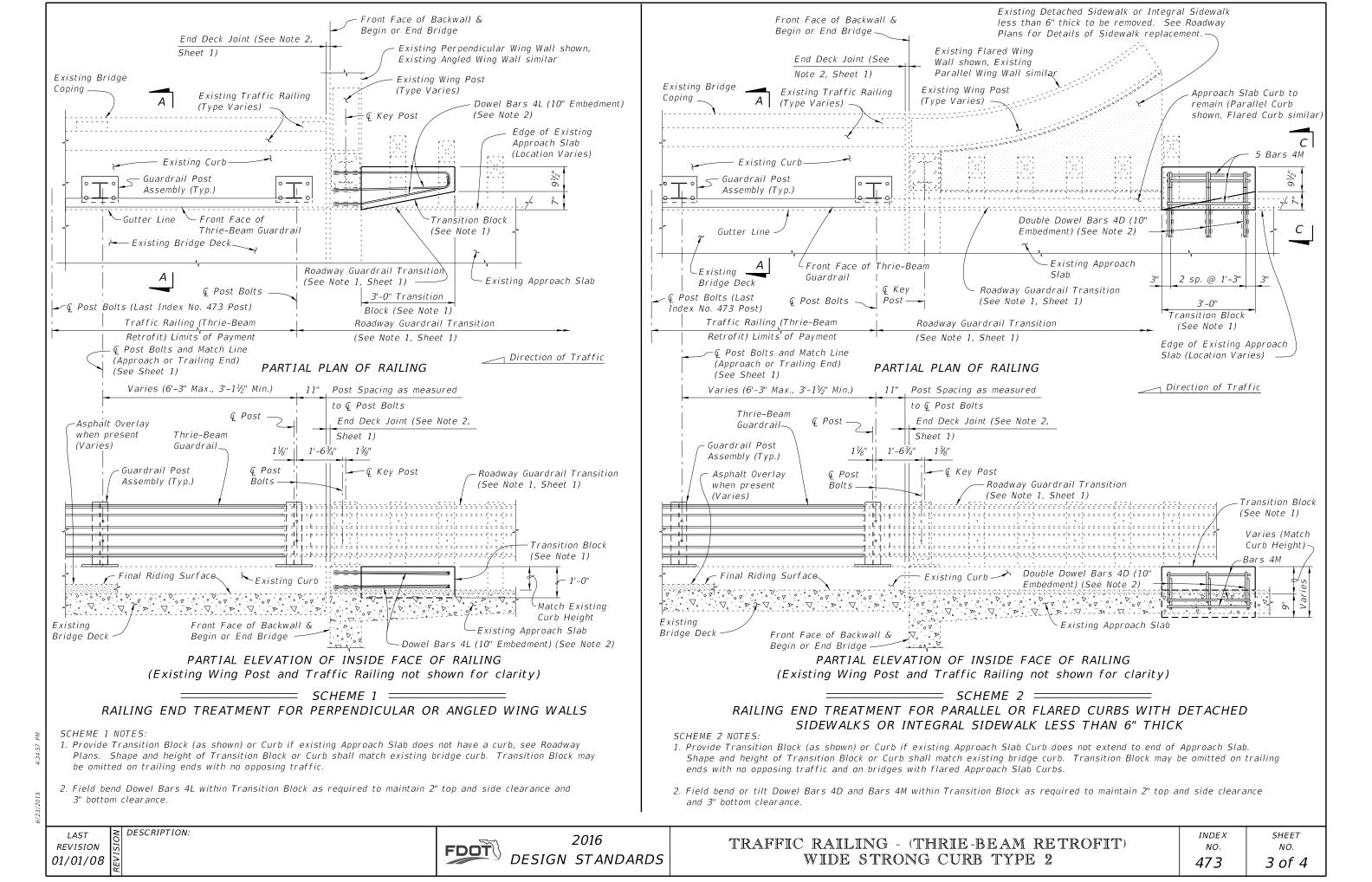
BILL OF REINFORCING STEEL BAR BENDING DIAGRAMS MARK SIZE LENGTH 1'-7½" D 4 3'-7" 4 4'-1" L ō М 4 2'-8" 2'-0<sup>1</sup>/2" DOWEL BAR 4D 3'-8'' 2'-8" BAR 4M DOWEL BAR 4L NOTE: All bar dimensions are out to out.

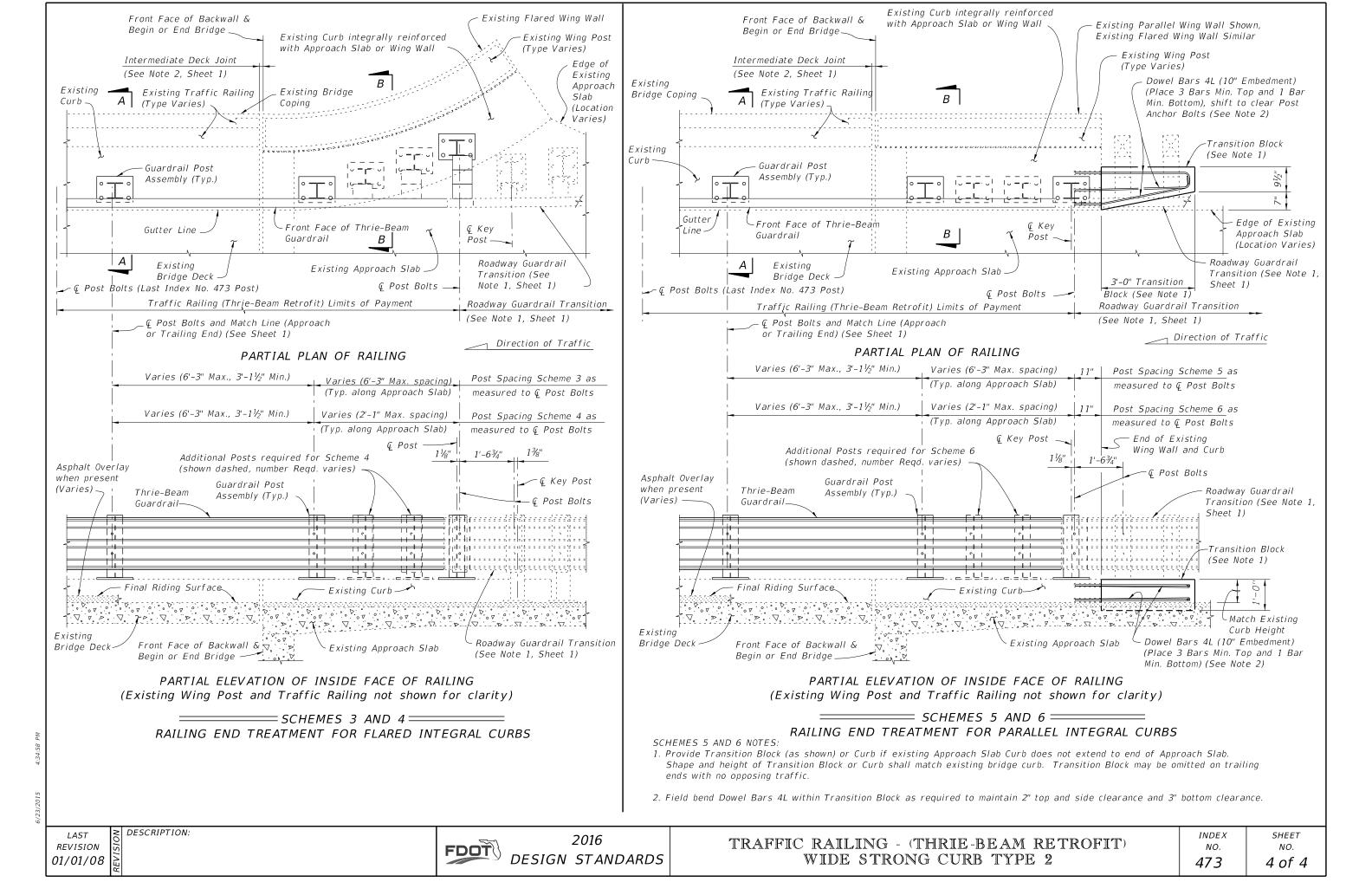


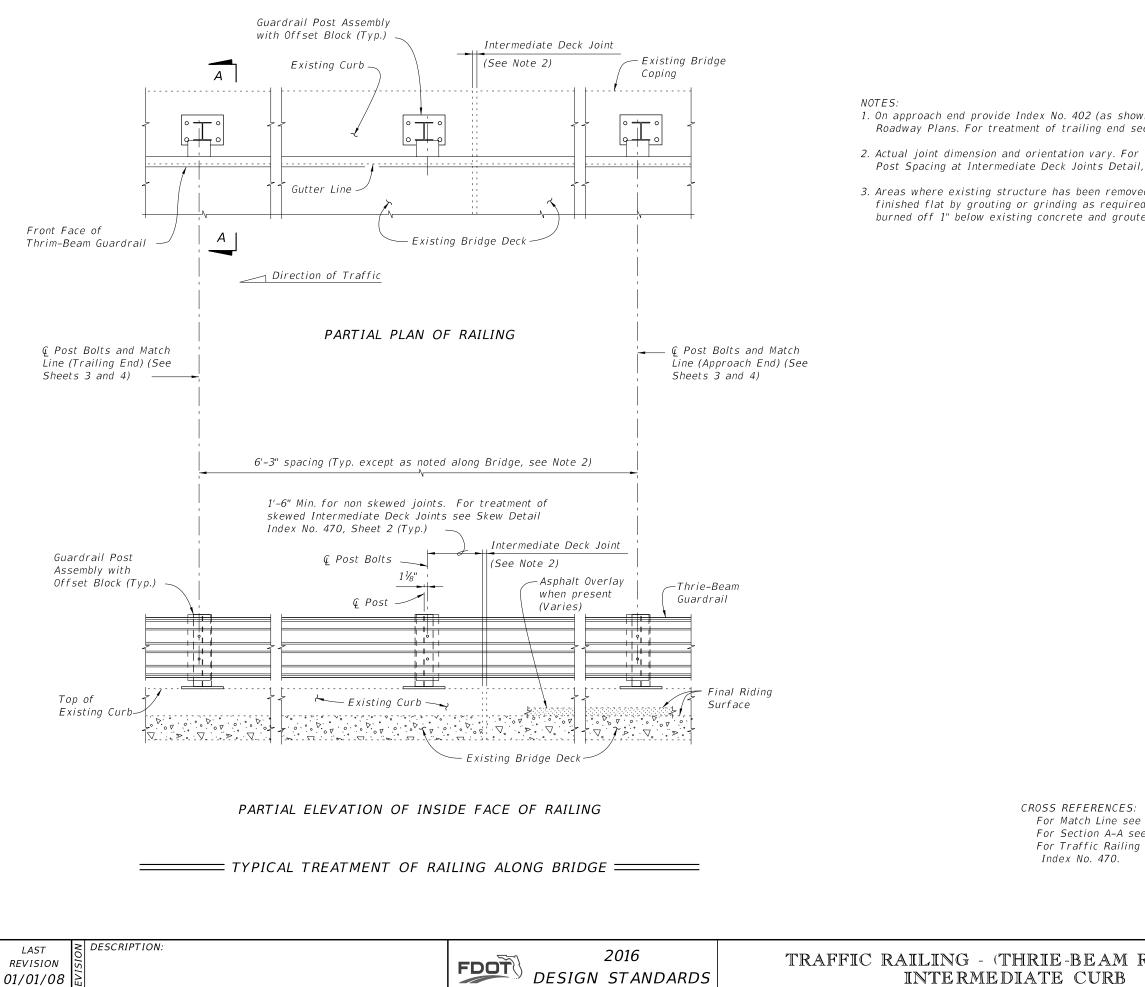
FDOT DESIGN STANDARDS TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2

Depth respectively).









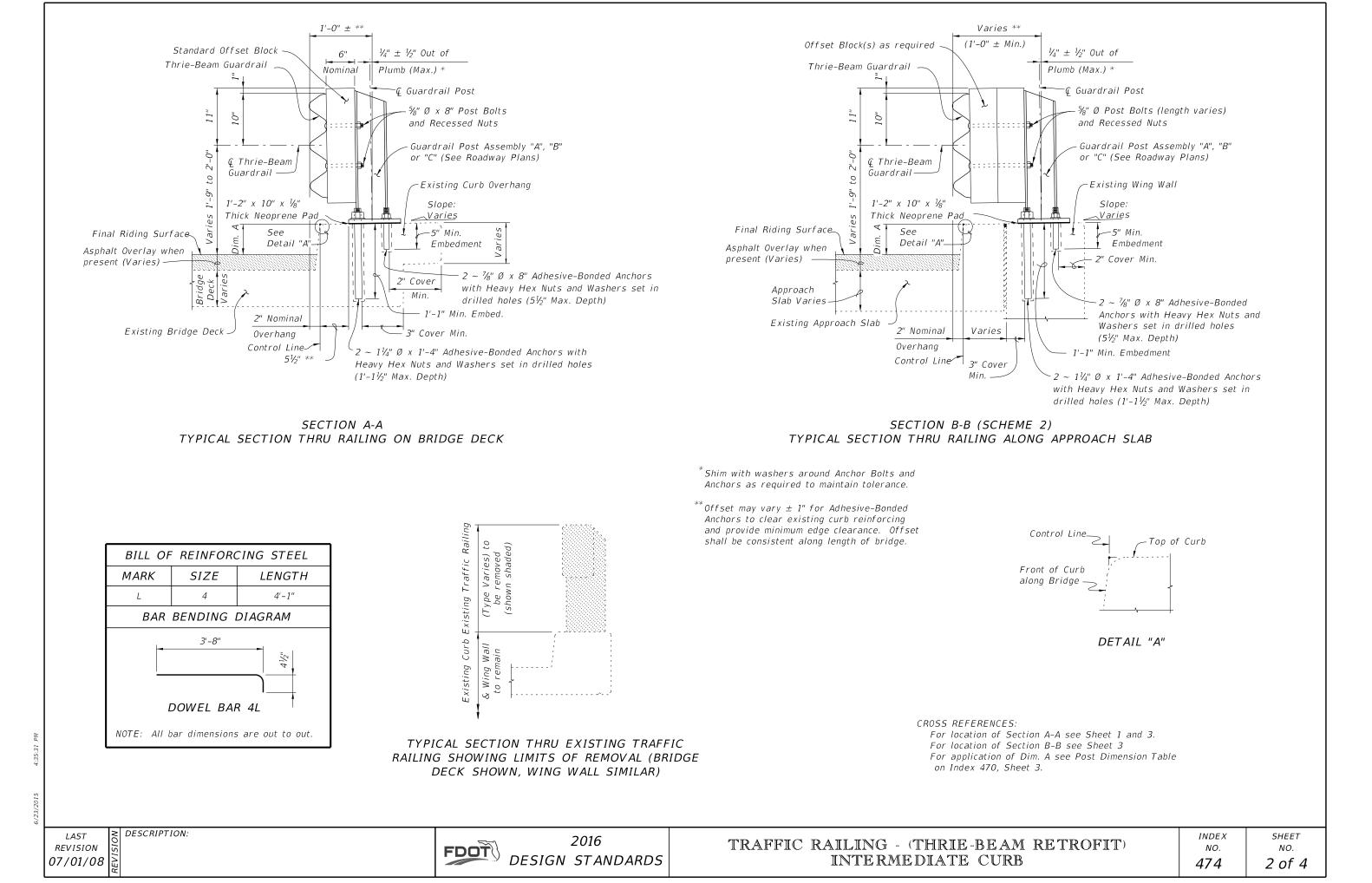
### NOTES:

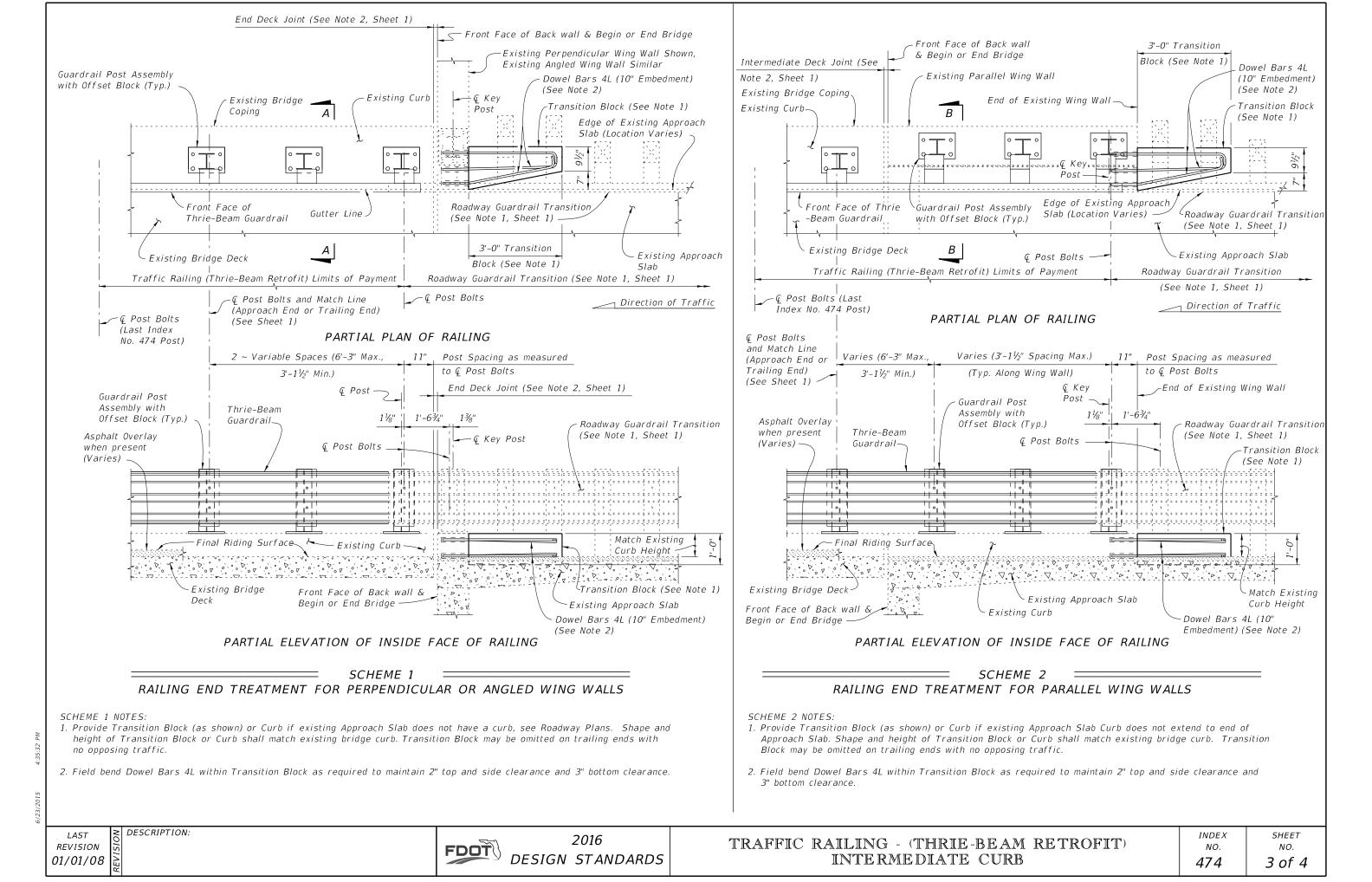
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

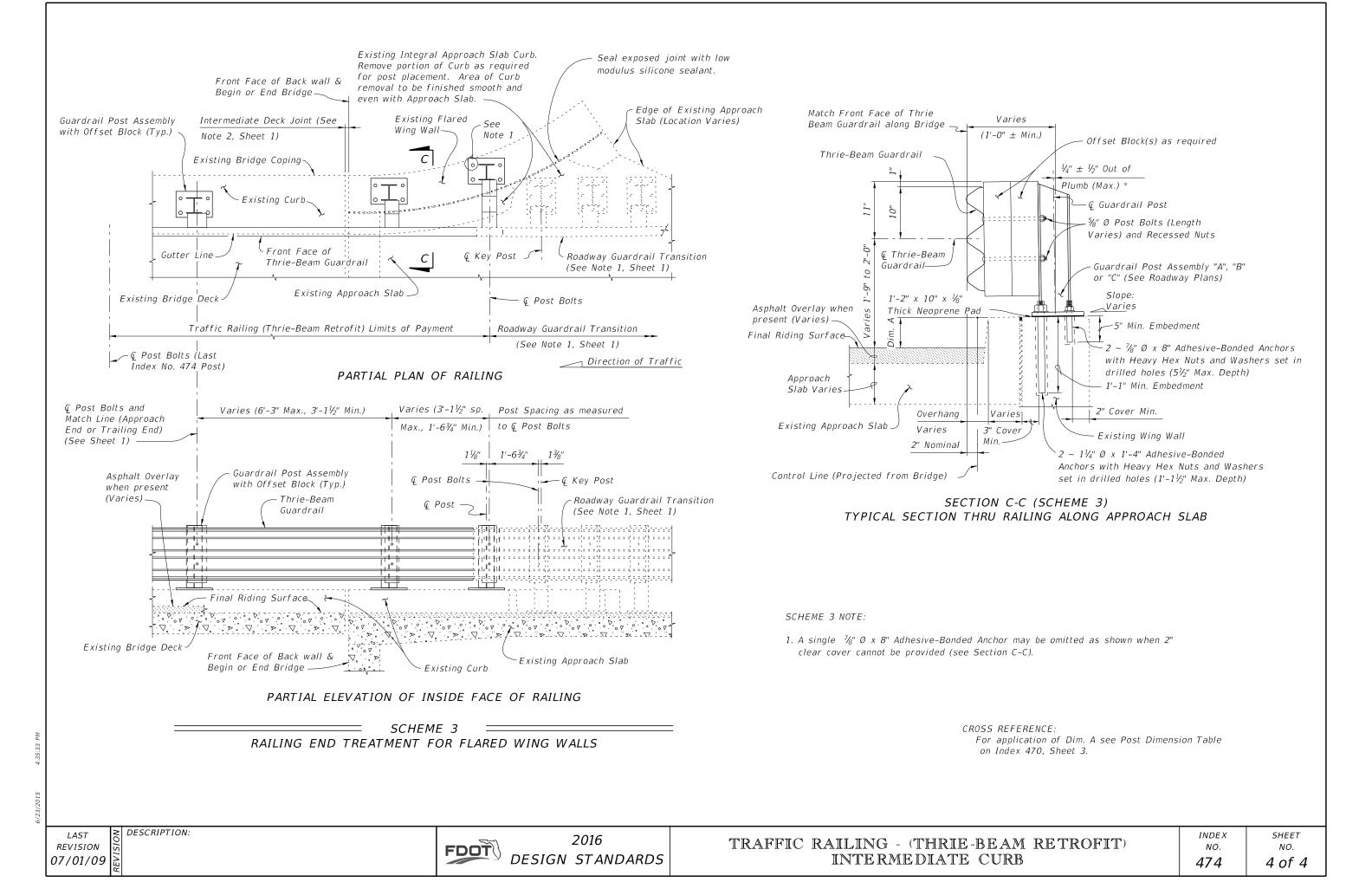
CROSS REFERENCES: For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

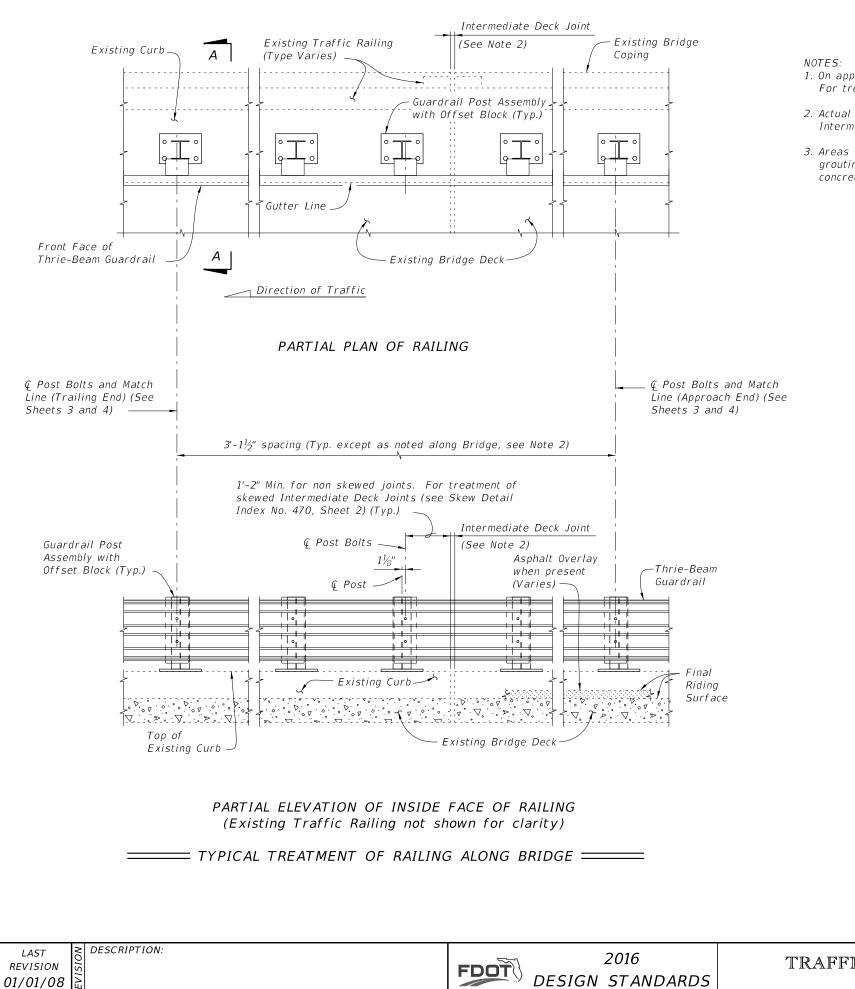
INTERMEDIATE CURB

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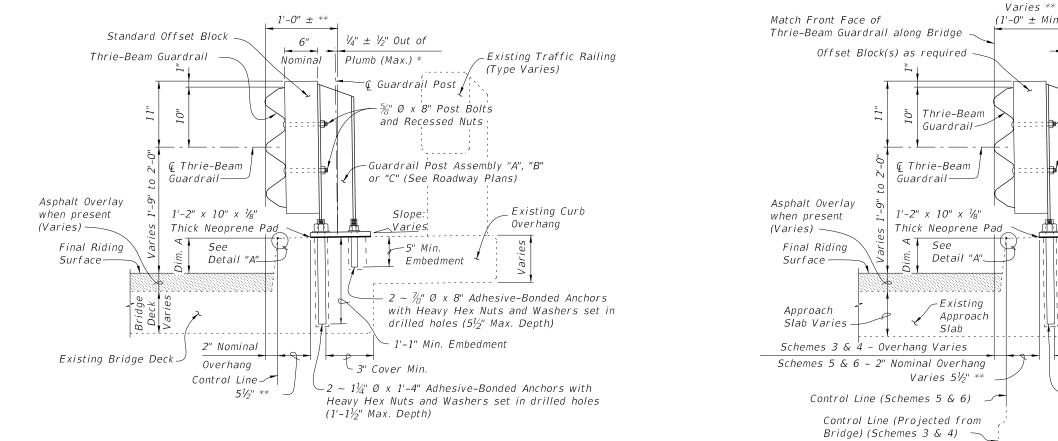




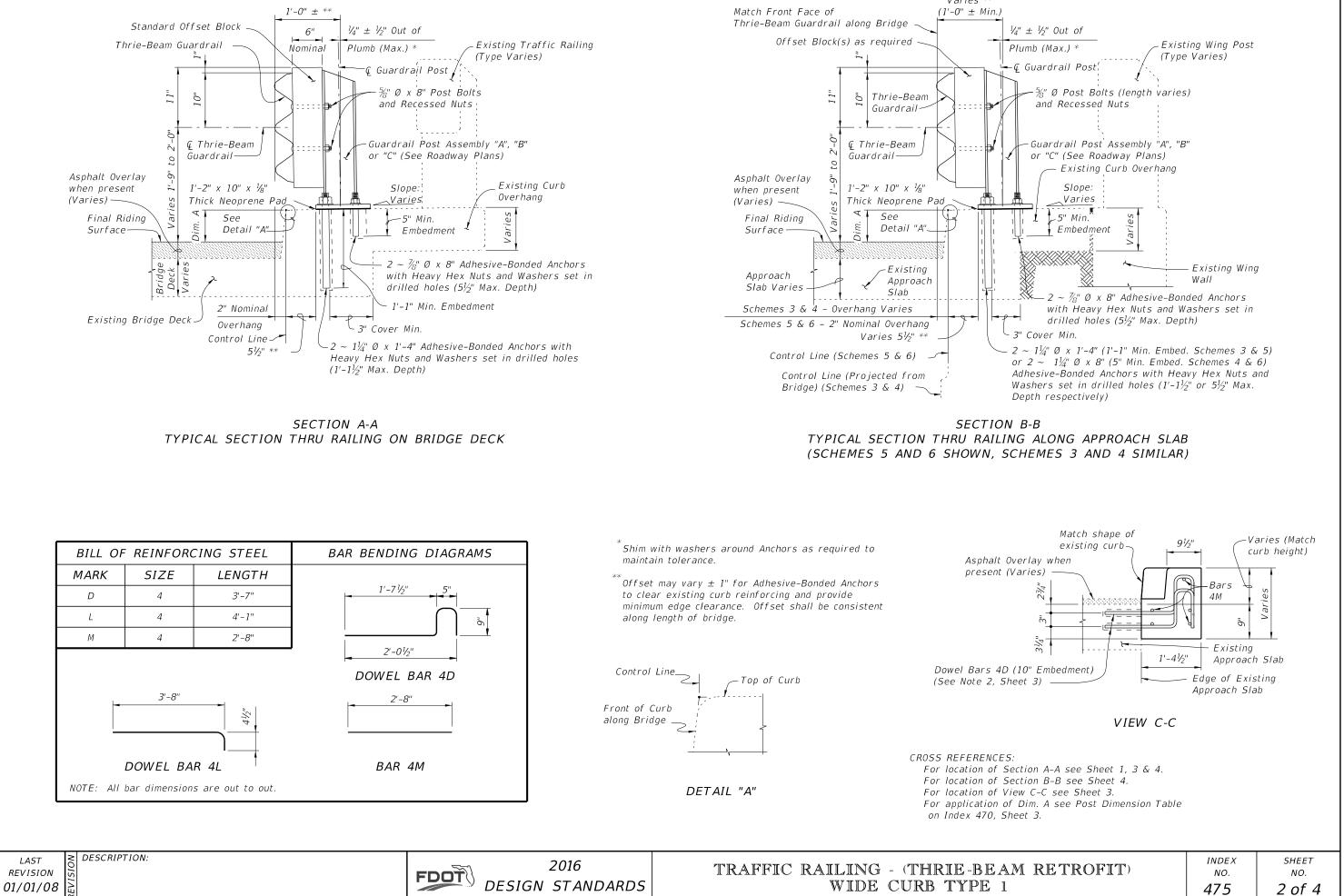
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

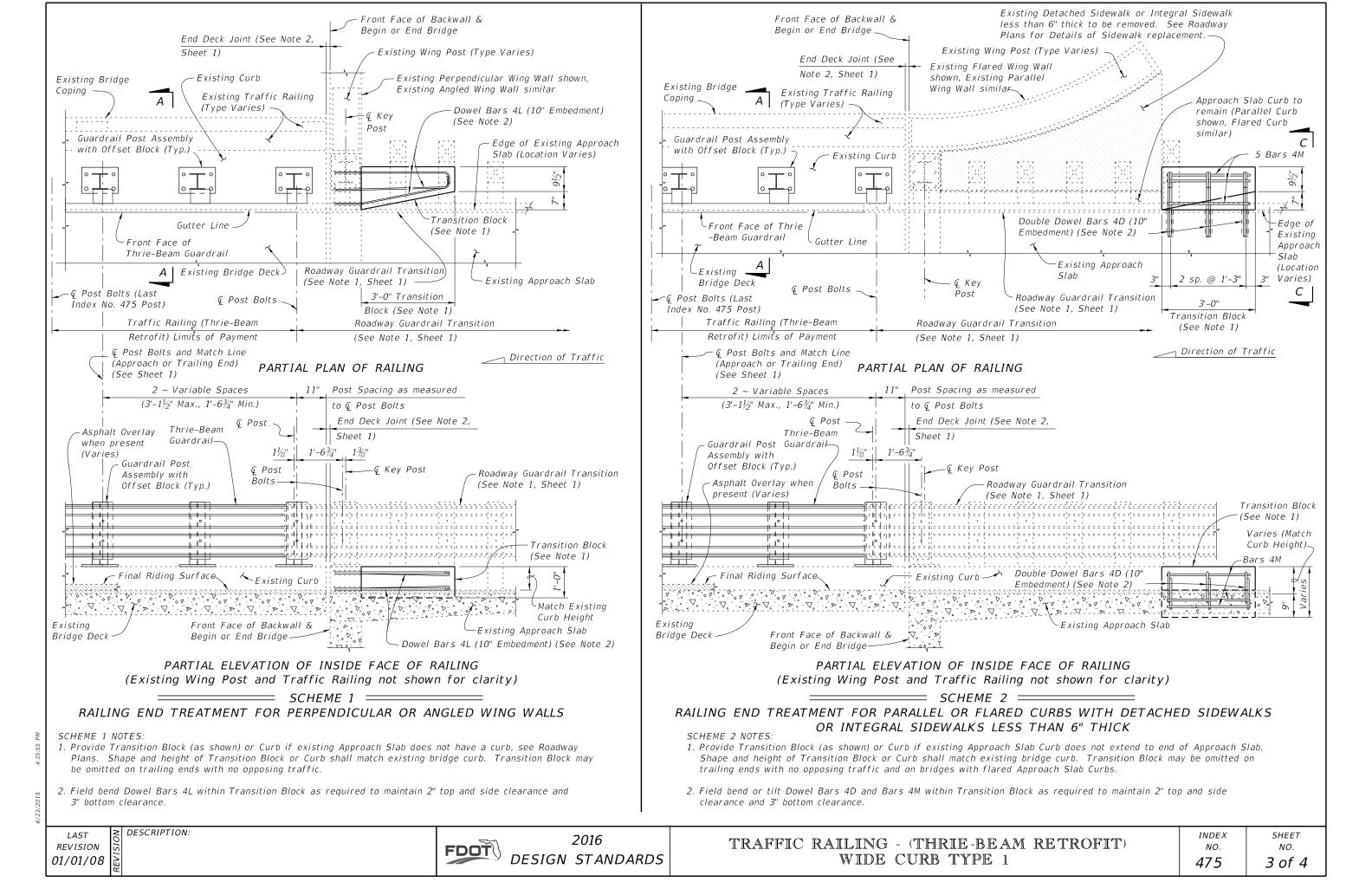
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

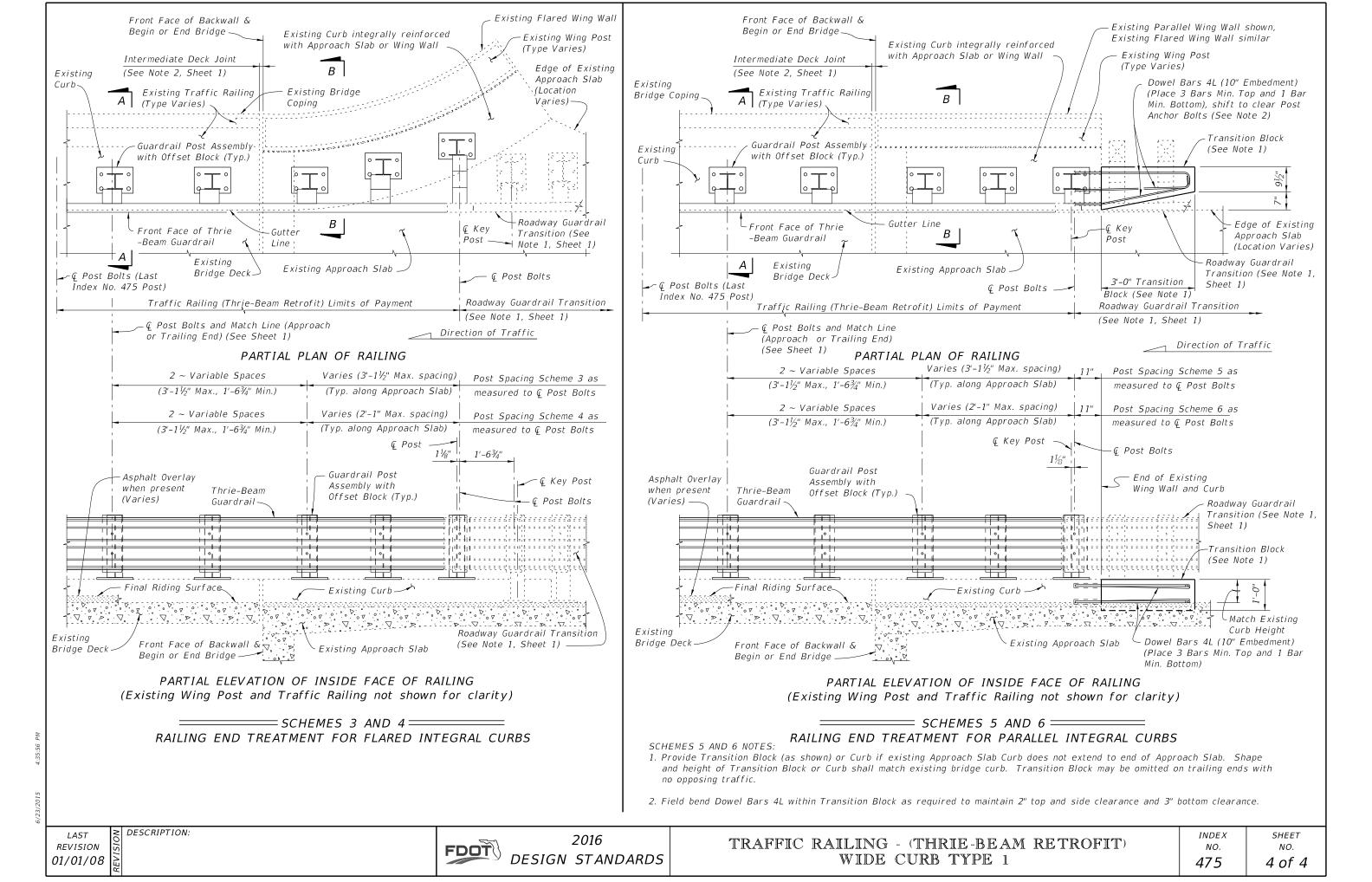
RETROFIT)	INDEX NO.	SHEET NO.
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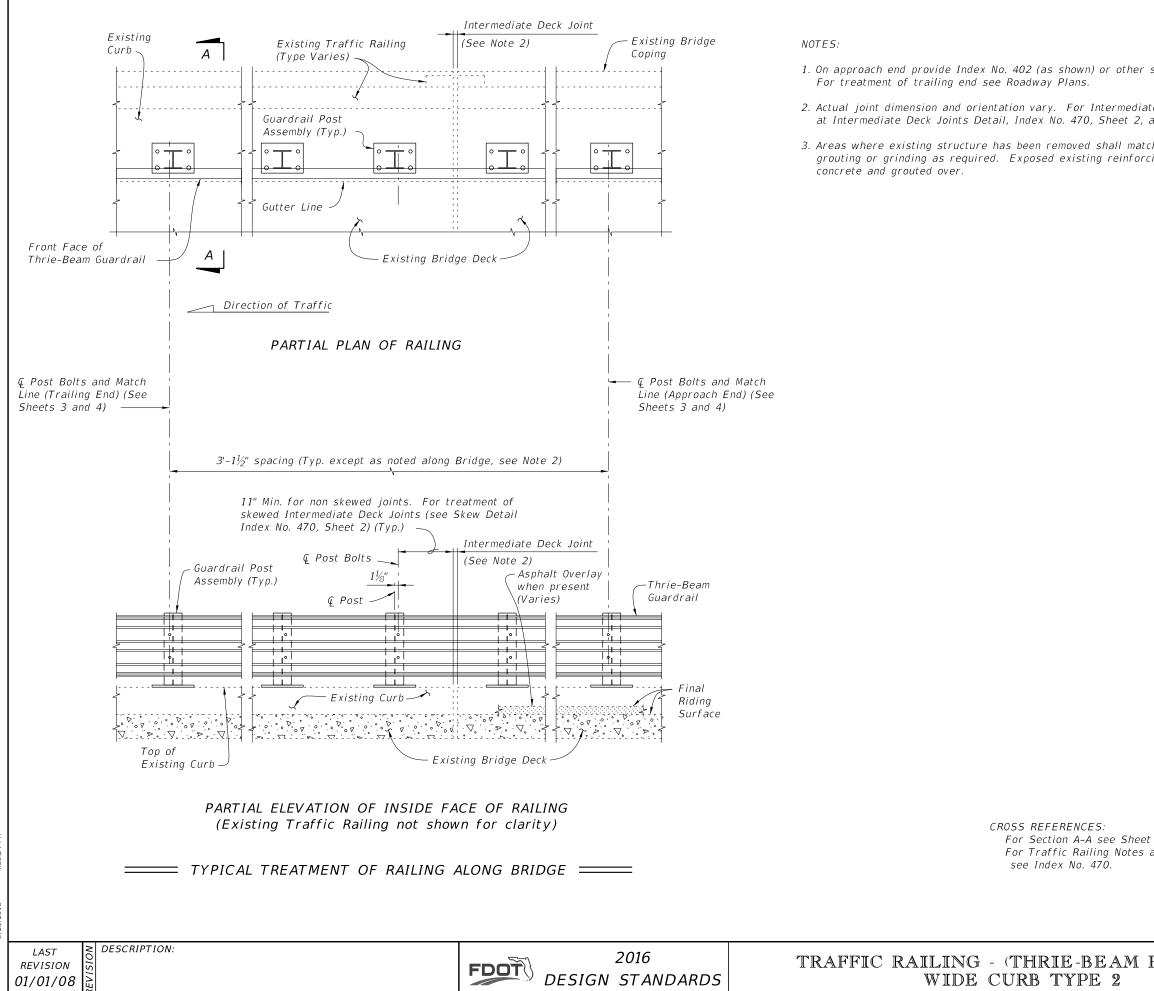


SECTION A-A









## NOTES:

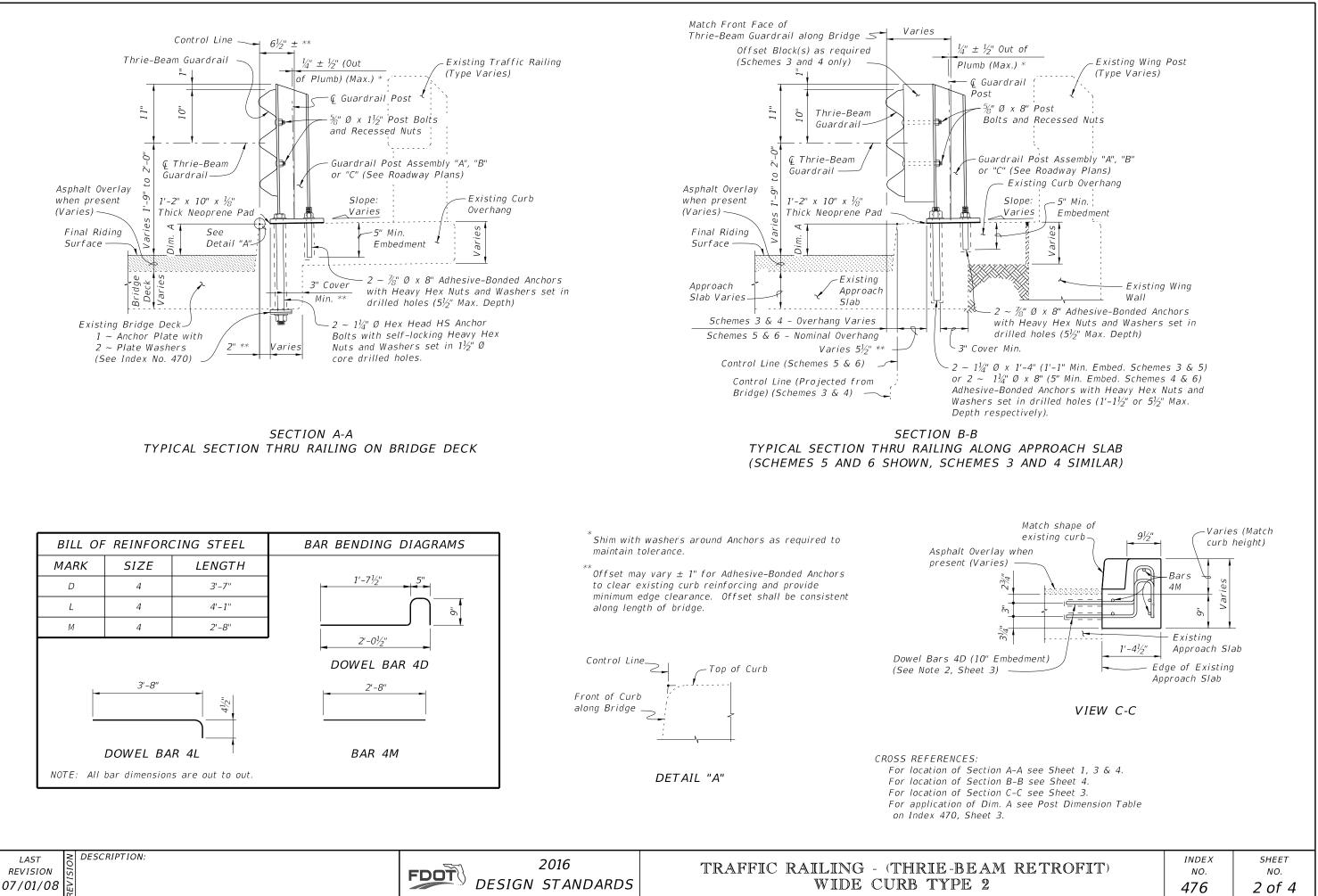
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet For Traffic Railing Notes an see Index No. 470.

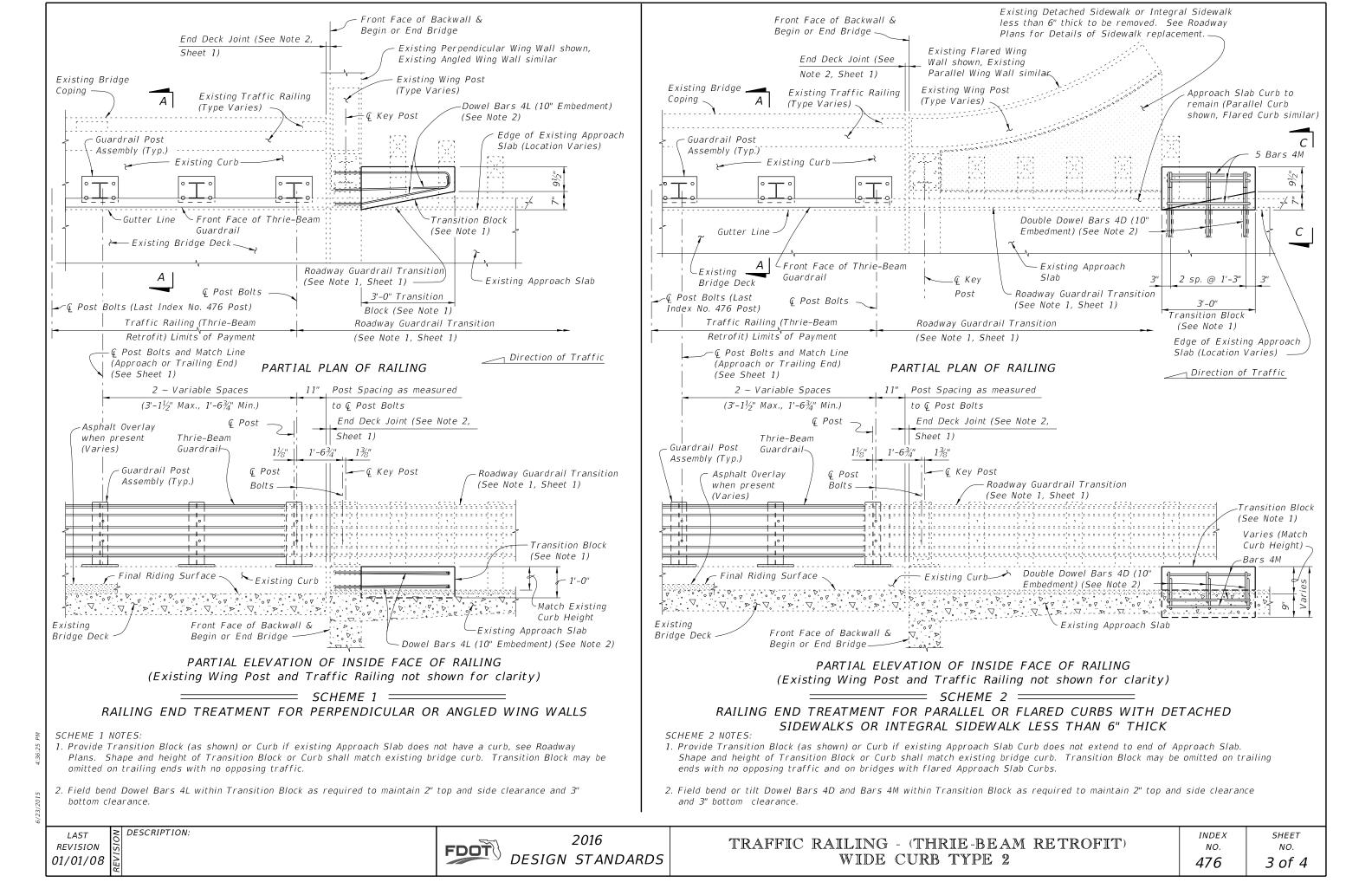
WIDE CURB TYPE 2

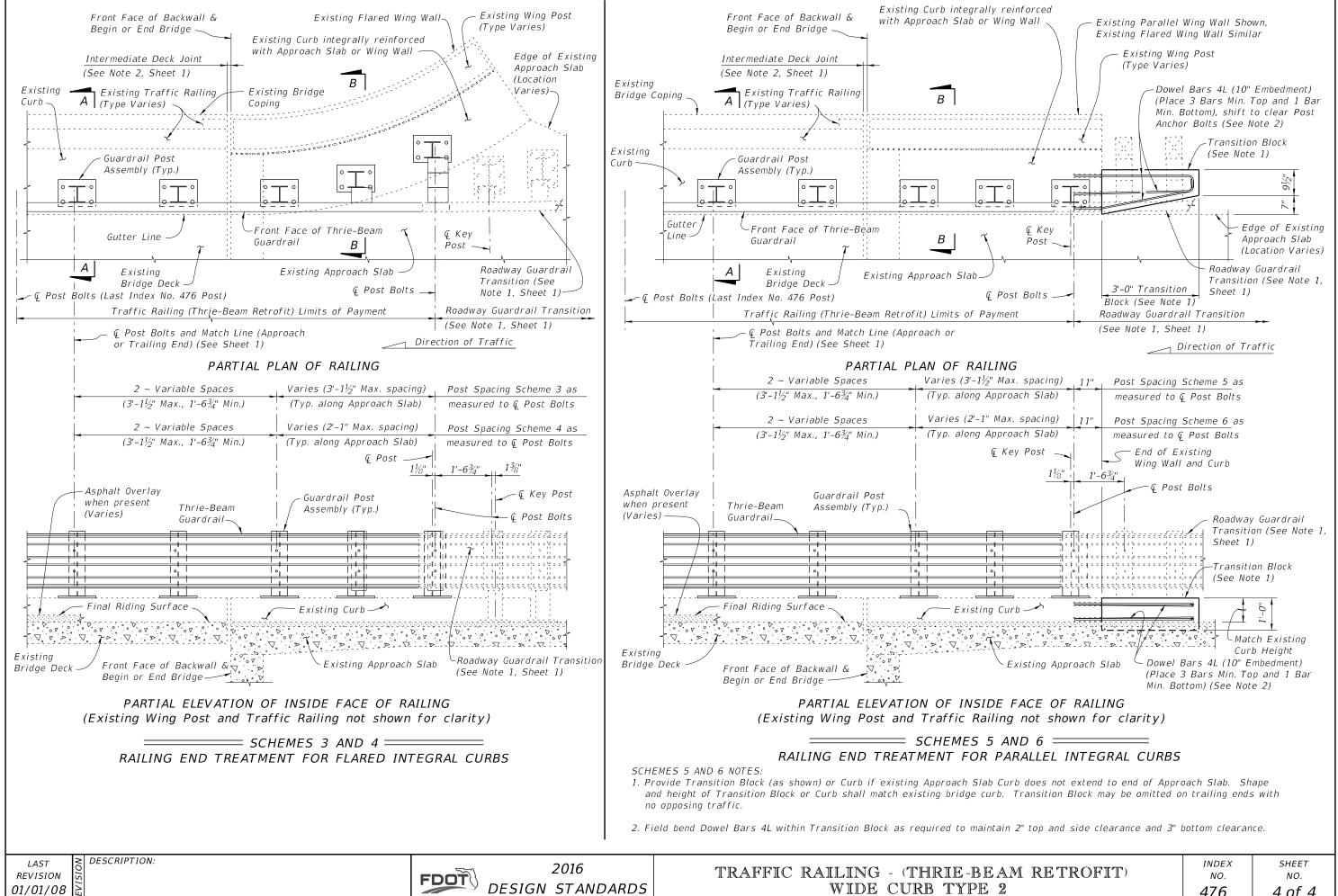
2.	
nd	Details

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### TRAFFIC RAILING RETROFIT NOTES

See Index 400 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be  $\frac{3}{4}$ " by  $2\frac{1}{2}$ " slotted holes.

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

COATINGS: All Nuts, Bolts, Anchors, and Washers shall be hot-dip galvanized in accordance with the Specifications.

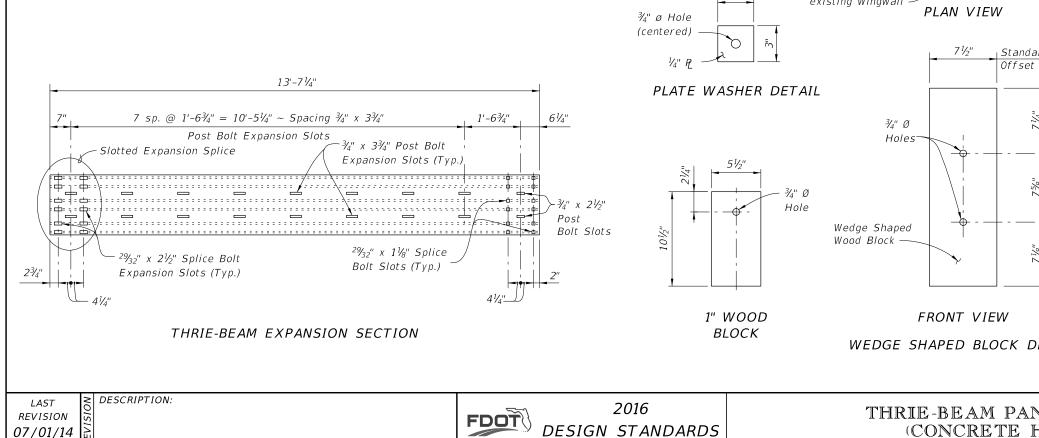
BRIDGES ON CURVED ALIGNMENTS: The details presented herein are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

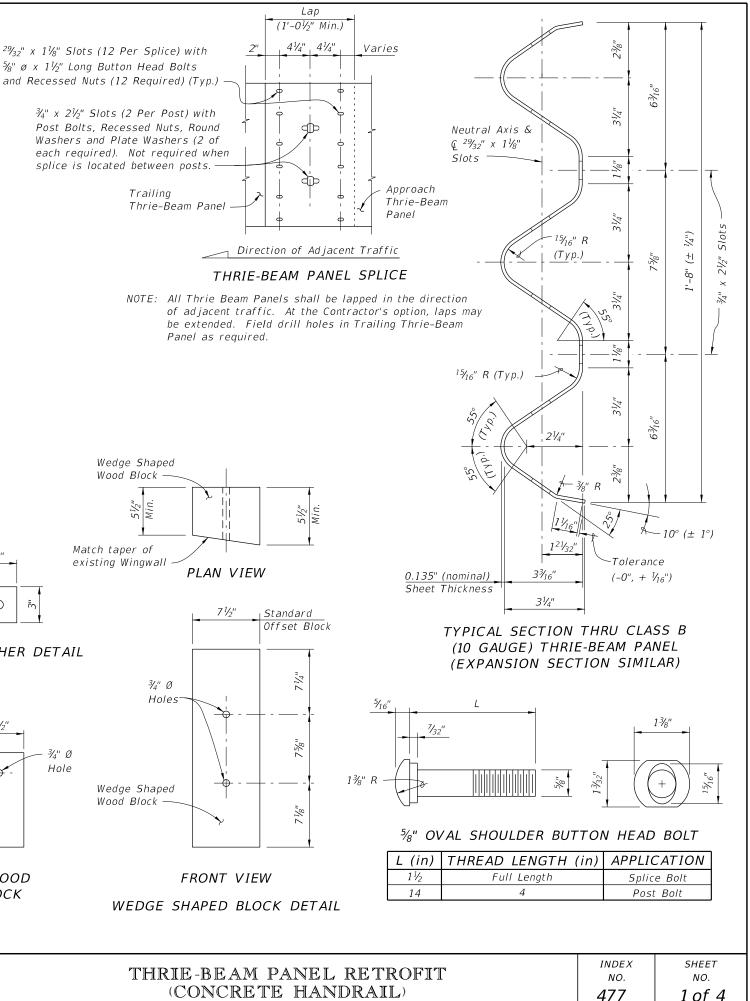
THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at 2<sup>1</sup>/<sub>2</sub>" slots in thrie-beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten bolts in  $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

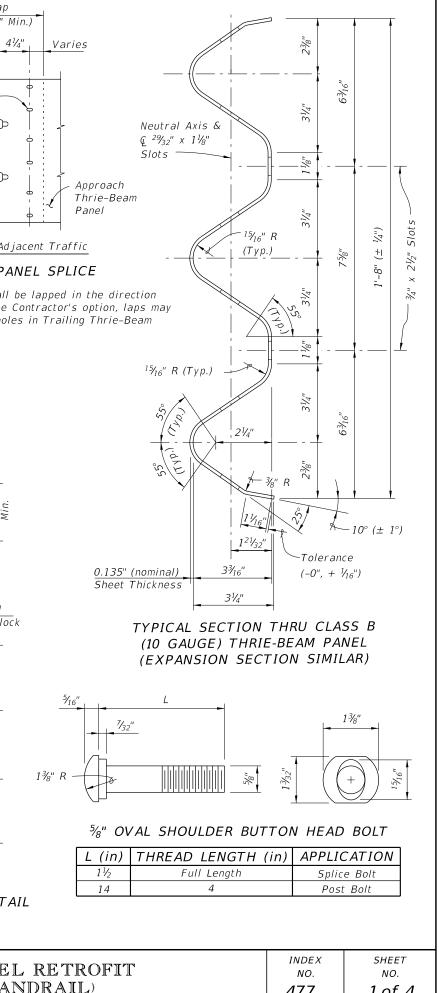
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered ( $\pm \frac{1}{4}$ ").

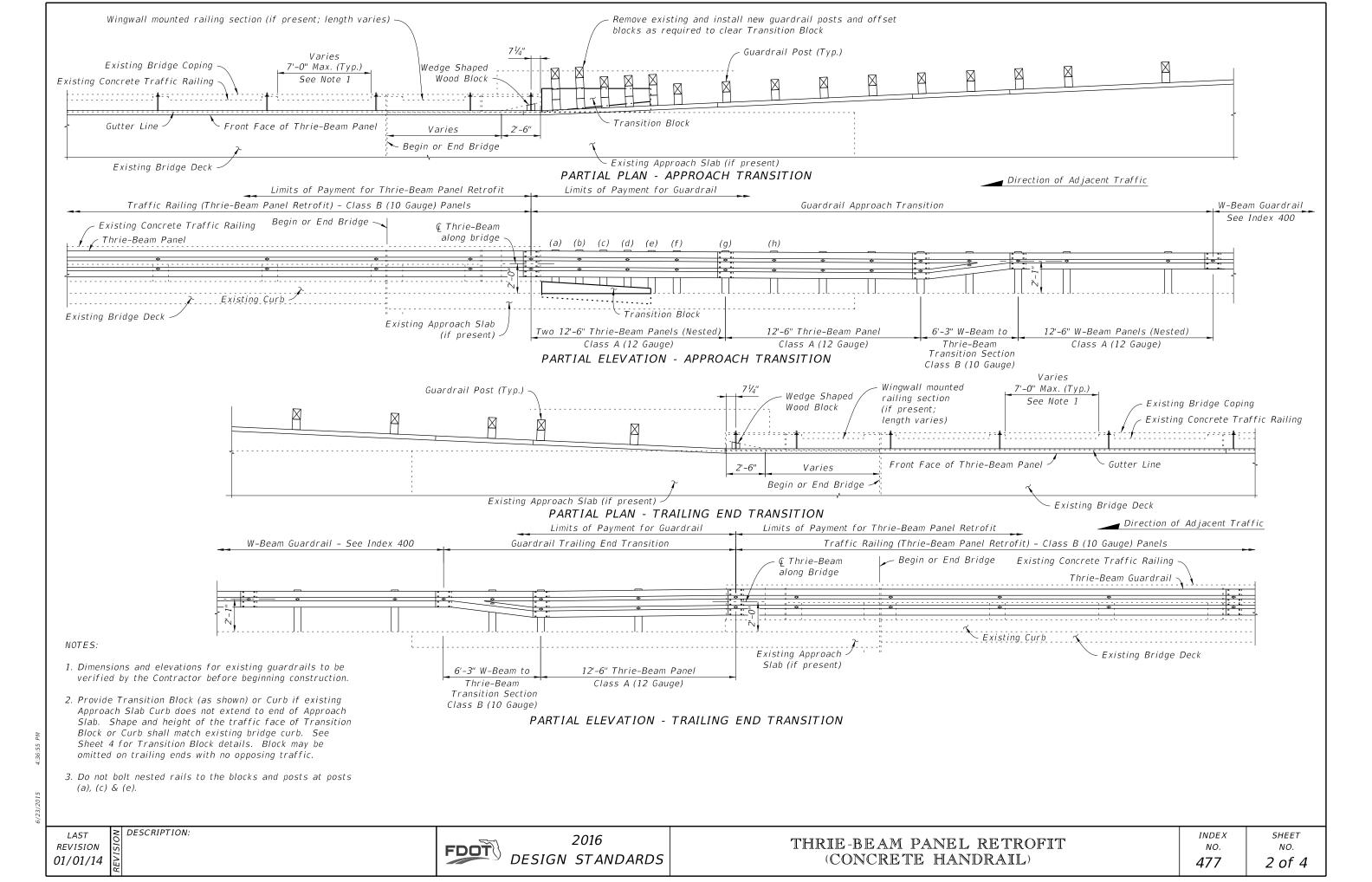
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie-Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

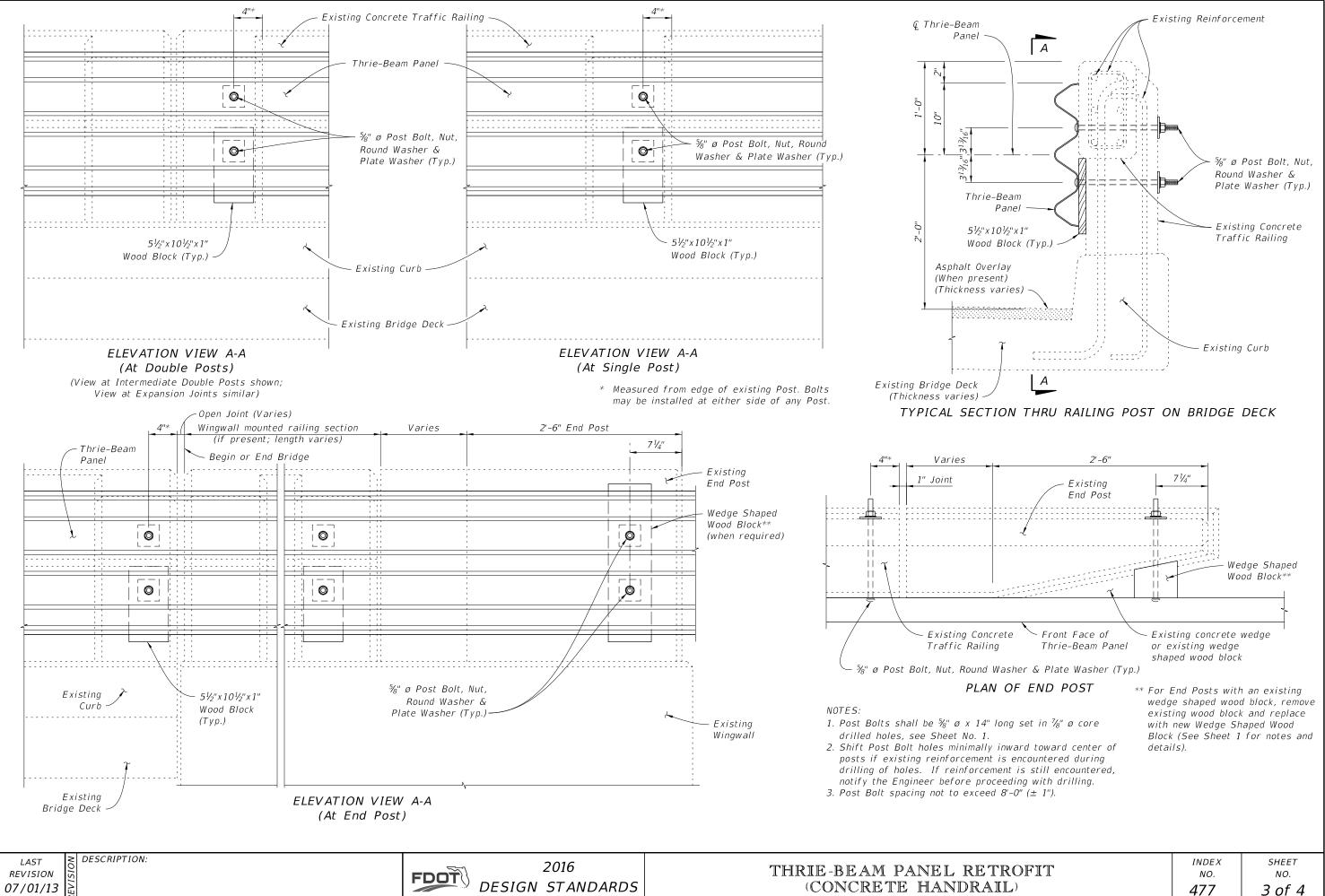
PAYMENT: Payment will be made under Thrie-Beam Panel Retrofit which shall include all materials and labor required to fabricate and install the retrofit railing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators, where required, will not be paid for directly but shall be considered incidental work.

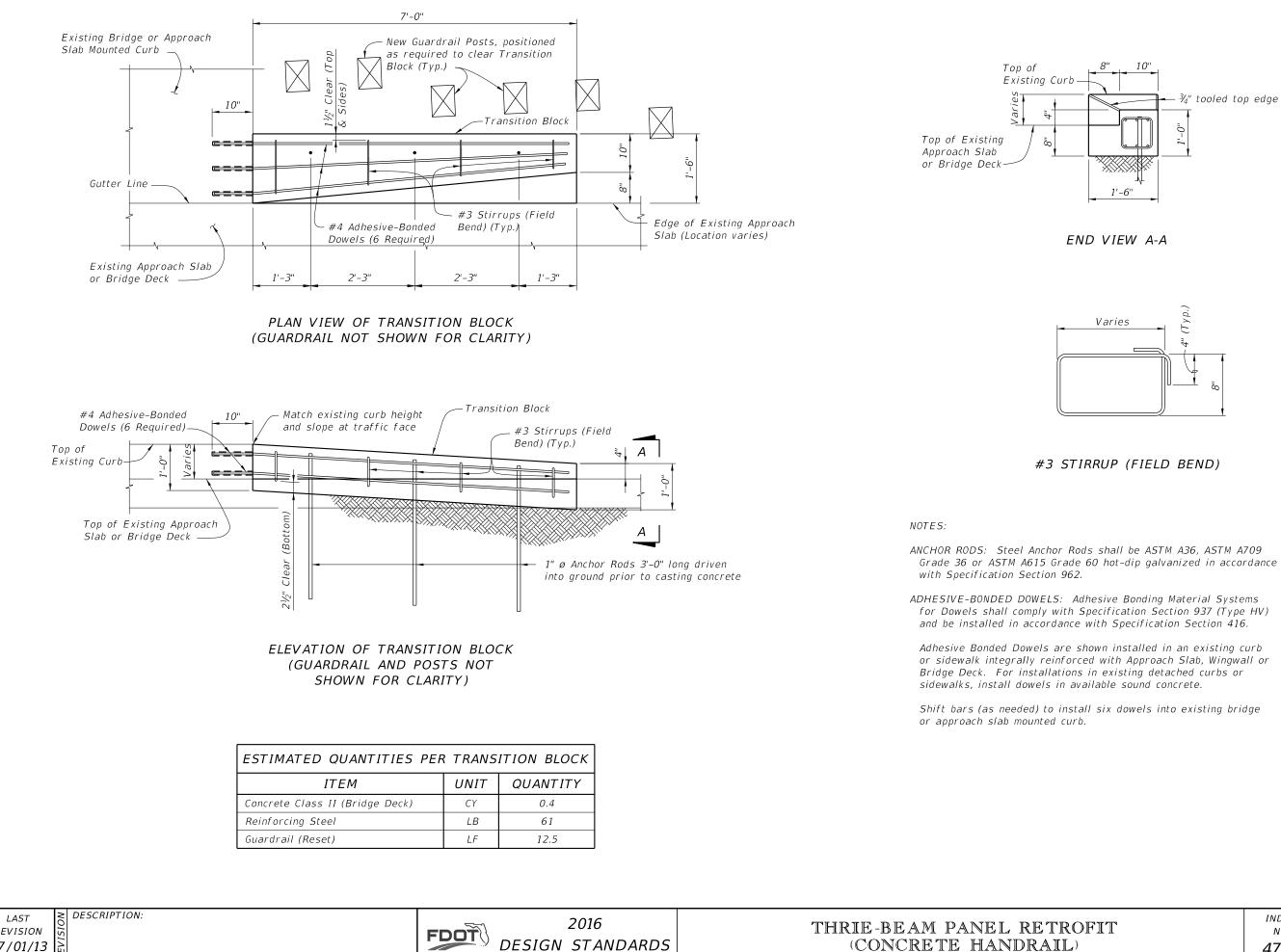








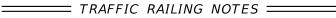




REVISION 07/01/13

(CONCRETE HANDRAIL)

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This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested previously and approved for a NCHRP Report 350 Test Level 4 rating, except for the Tapered End Transition on Index No. 484.

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

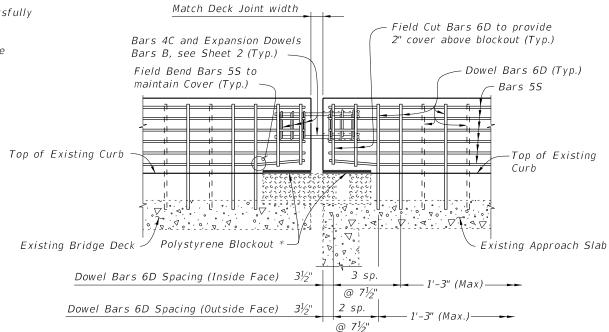
EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-O" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

- BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.
- NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or  $\frac{3}{6}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures. ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

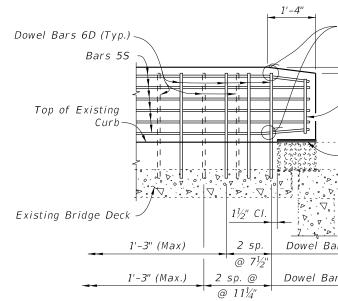
BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table below. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) includes all materials and labor required to construct the railing and incidental work as required for transition blocks, curbs, spread footing approaches, and Barrier Delineators.



# PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT - SCHEMES 2 THRU 5 (Begin or End Bridge Shown, Intermediate Joints Similar)

\* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWIN JOINT AT BEGIN OR END BRID (Guardrail Transition not show

Limiting Station of Transition (See Roadway Plans) (Min.) (2'-6" Min.) ........... ...... \*\*=\*\*\*\*\*\* . . . . . . NAME OR DATE . . . . . . . . . . . . . . . . BRIDGE NUMBER \*\*=========== . \_\_\_\_\_ \_\_\_\_ Top of Existing Curb ..... NAME, DATE AND BRIDGE NUMBER LETTERING DETAIL

1'-0''

Varies

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT QUAN 9" Curb	QUANTITY	
		Increment	
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index No. 484, Sheet 4 for Spread Footing Approach Quantities.

2016

BARRIER DELINEATOR

SPACING

Spacing (Ft.)

40'

80'

None Required

Distance -

Edge of Travel Lane

4' to 8'

> than 8'

to Face of Railing < 4'

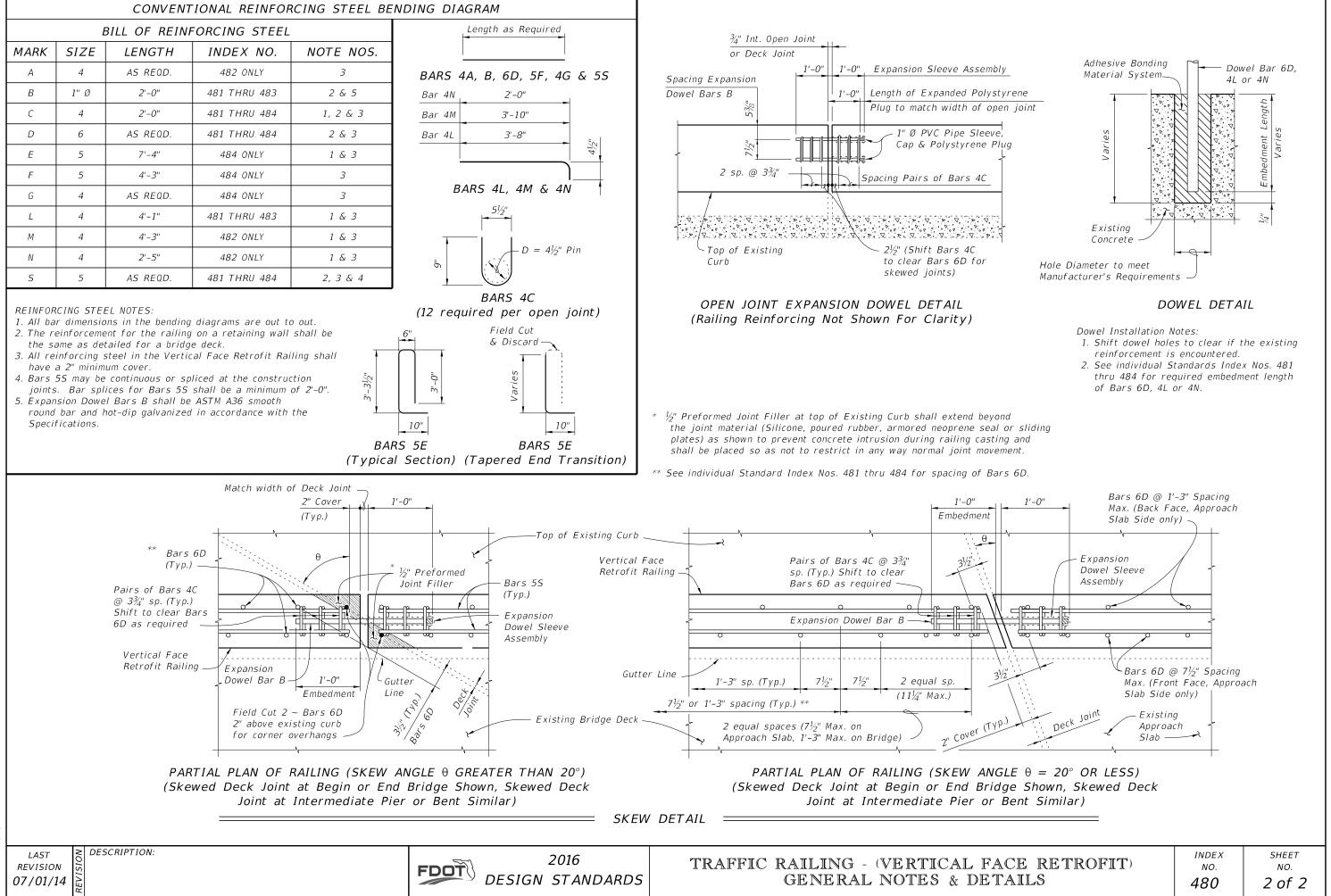
DESCRIPTION: LAST REVISION

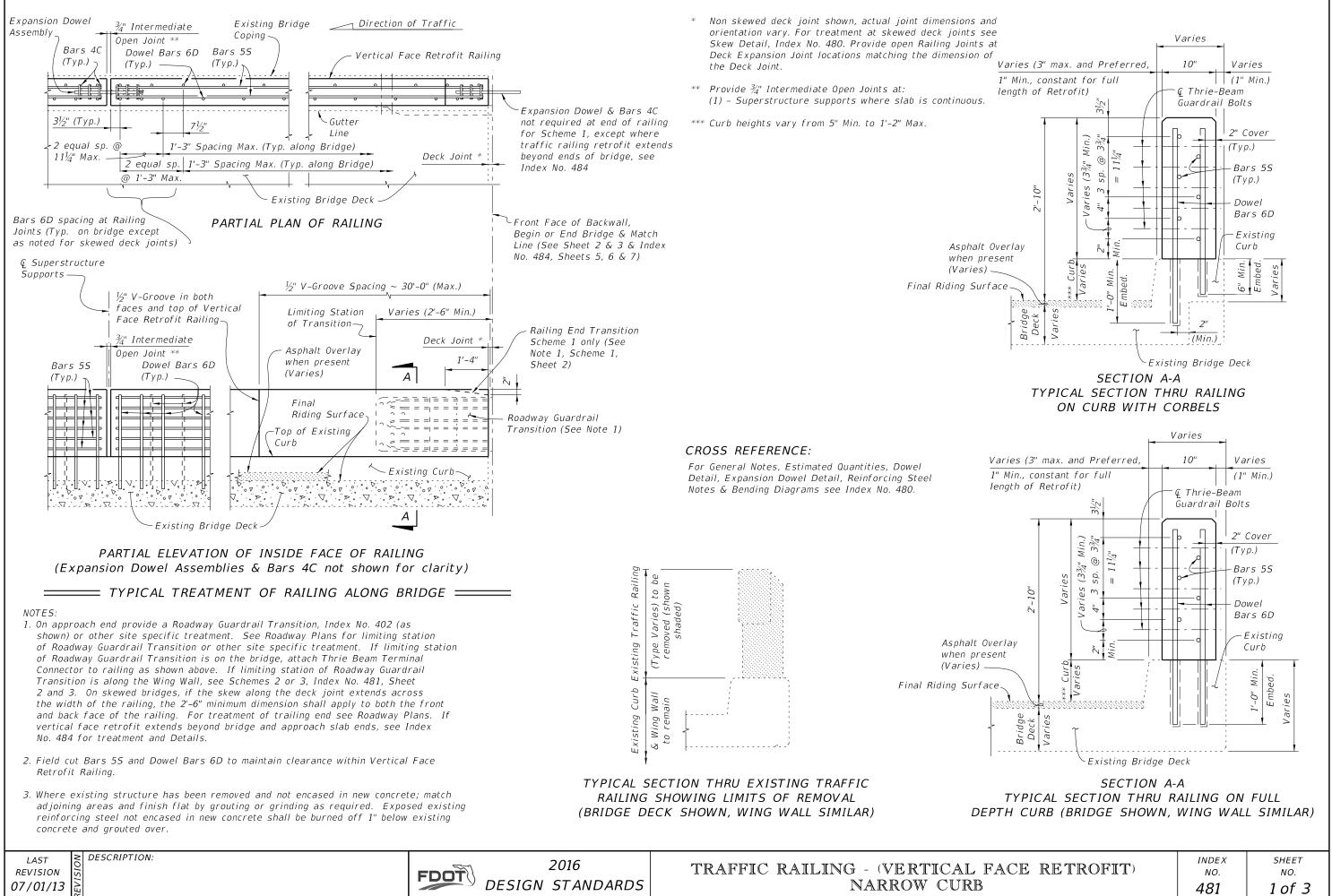
07/01/14

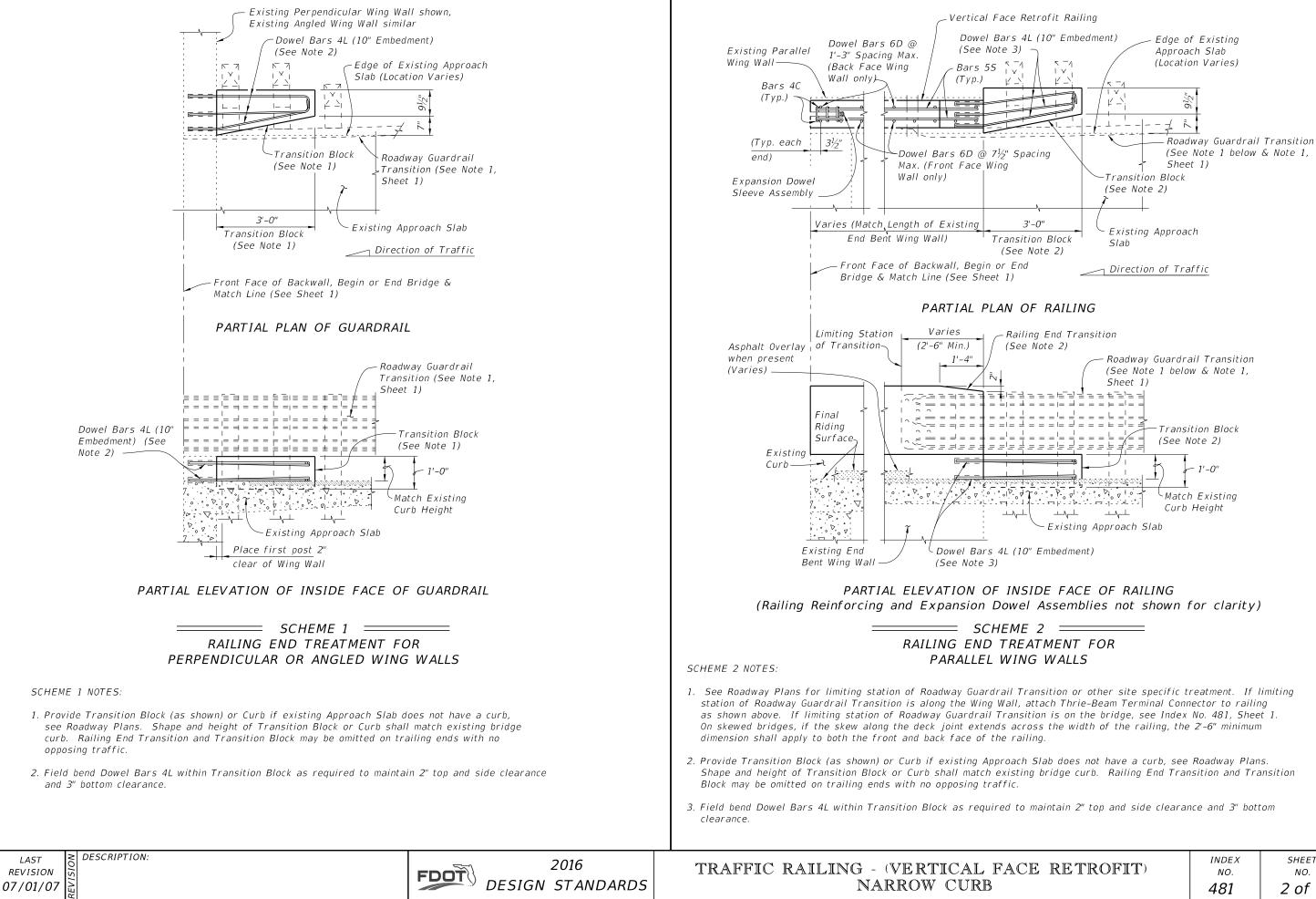


TRAFFIC RAILING - (VERTICAL FACE GENERAL NOTES & DETAII

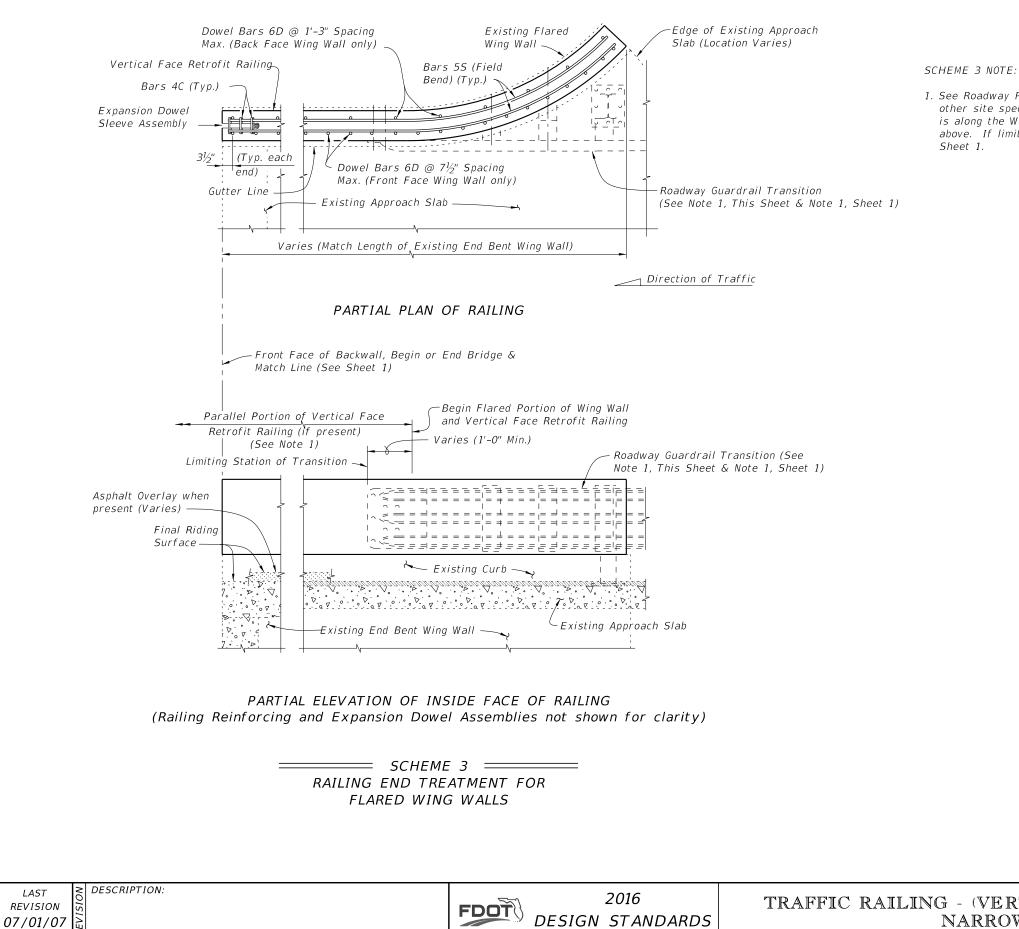
Field Bend Bars 5S to maintain clearance		
N Field Cut Bars 6D 2" cover above bloc		
Top of Existing (	Curb	
– Polystyrene Blockout *	-	
$\nabla_{\mathbf{v}} \circ (\nabla_{\mathbf{v}} \circ \nabla_{\mathbf{v}} \circ \nabla_{\mathbf{v}}$		
Existing Approach SI	ab	
rs 6D Spacing (Inside Face)		
rs 6D Spacing (Outside Face)		
IG FINGER/SLIDING PL DGE - SCHEME 1 wn for clarity)	ATE	
RETROFIT) LS	INDEX NO. <b>480</b>	<sup>SHEET</sup> NO. <b>1 of 2</b>







RETROFIT)	INDEX NO.	SHEET NO.
	481	2 of 3



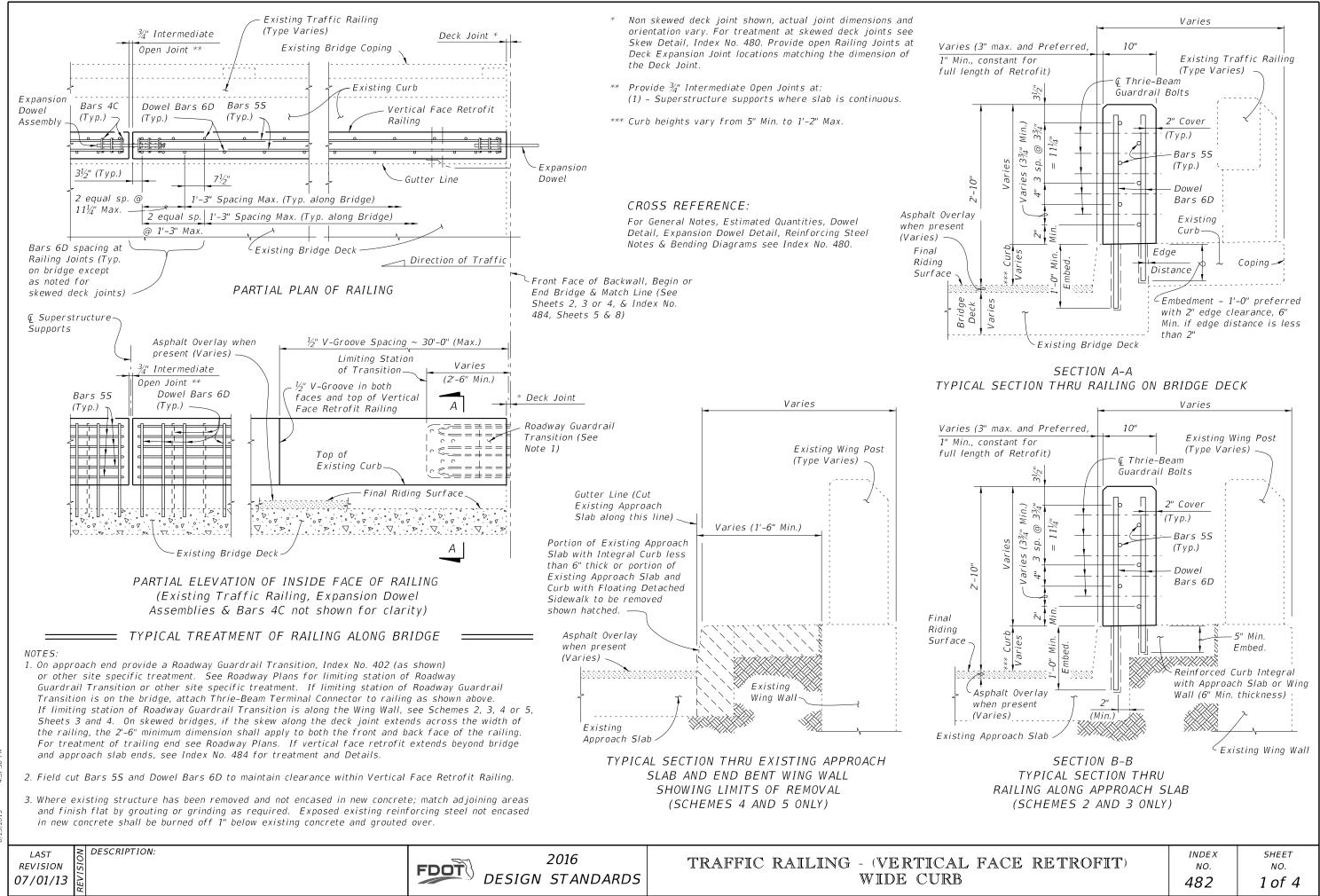
LAST

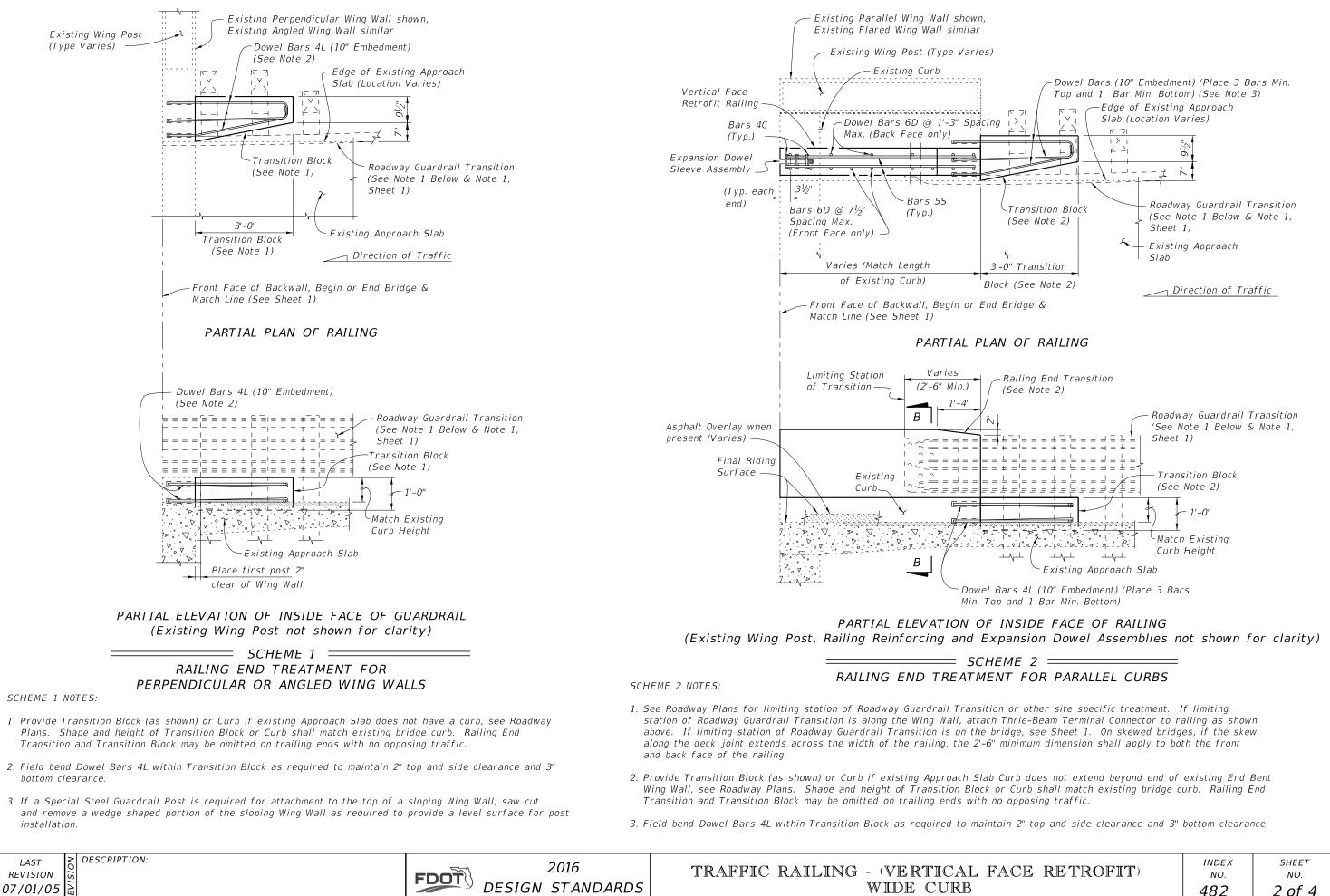
REVISION

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or

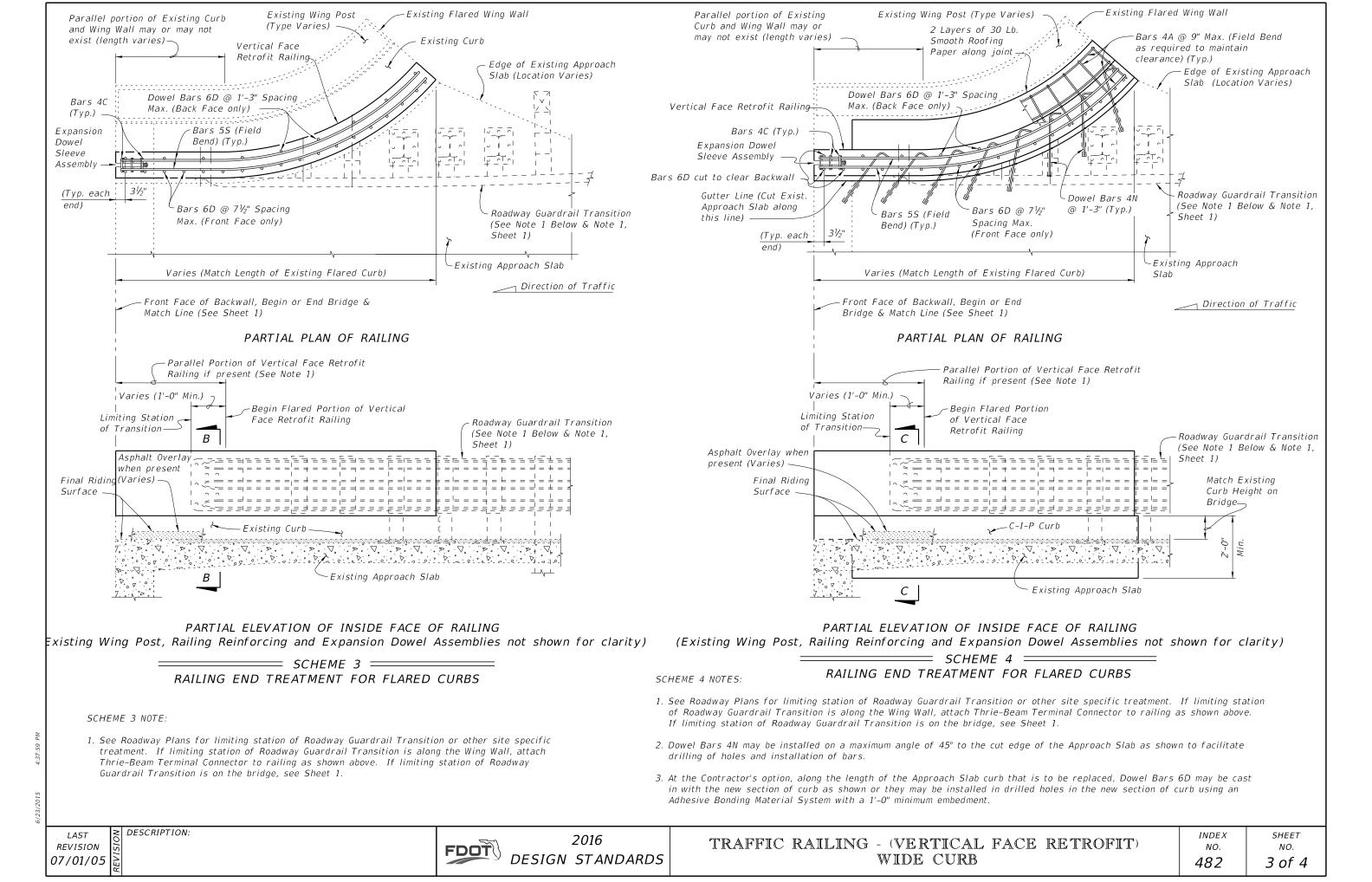
other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see

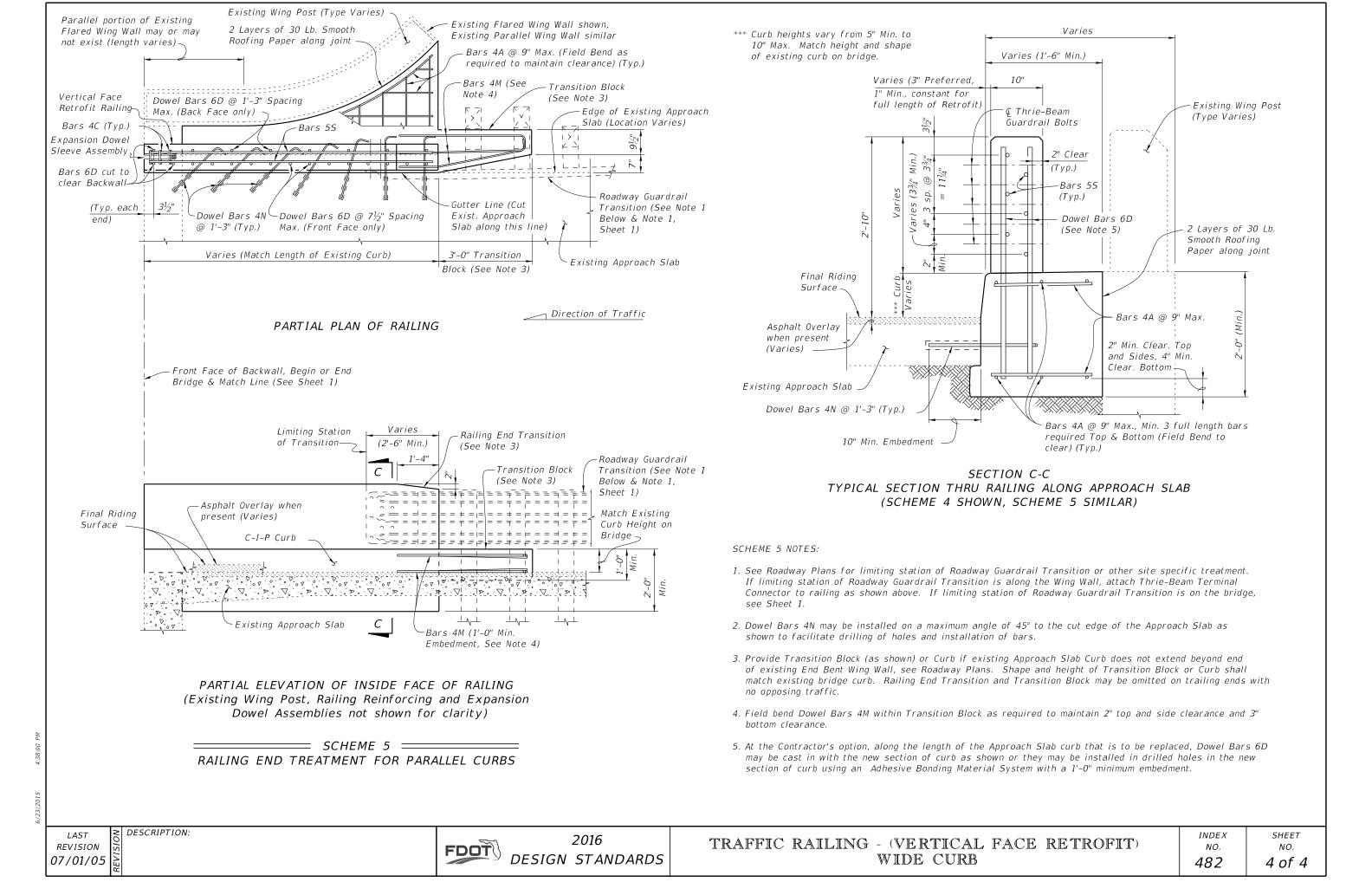
RETROFIT)	INDEX NO.	SHEET NO.
	481	3 of 3

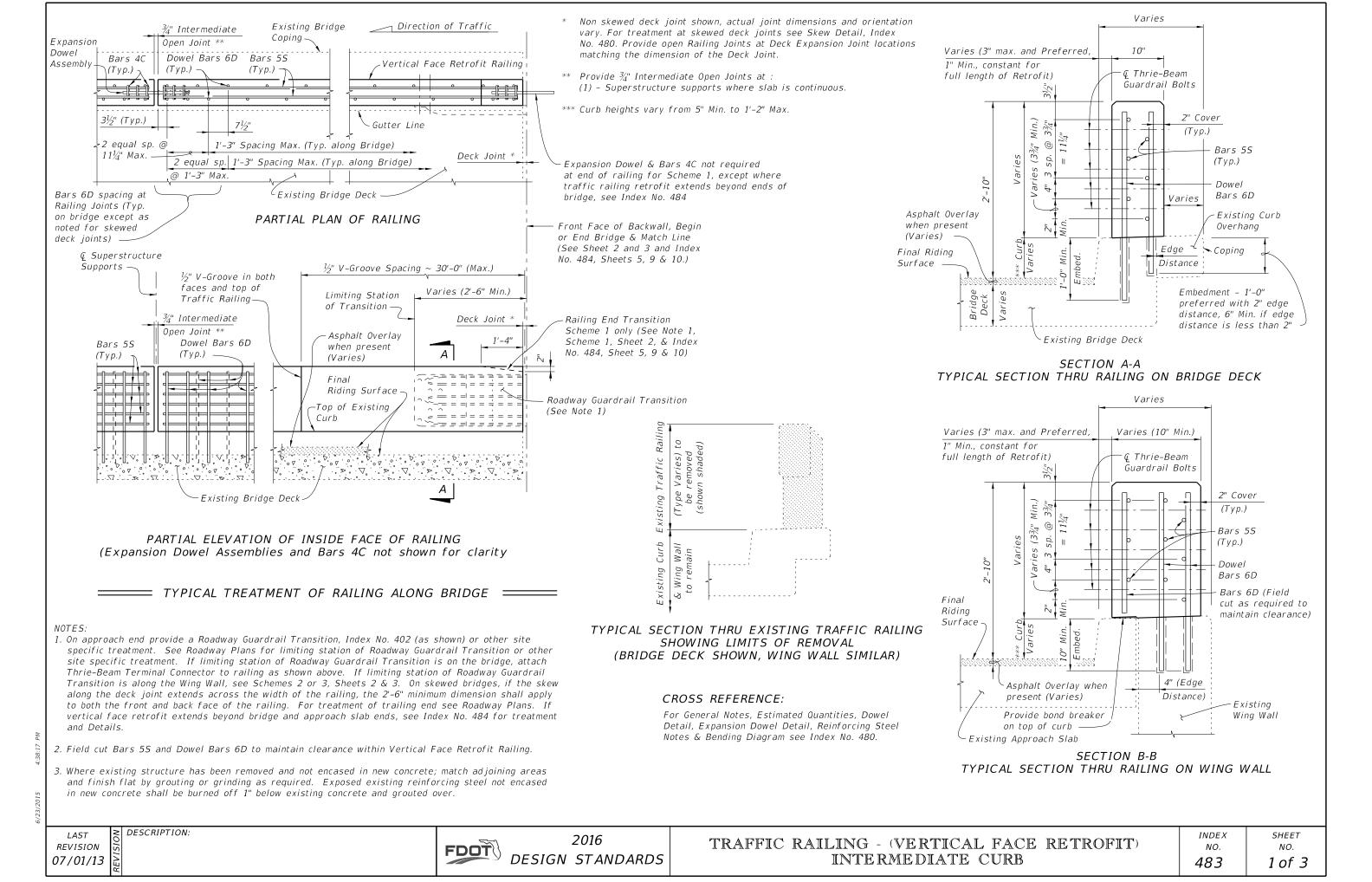


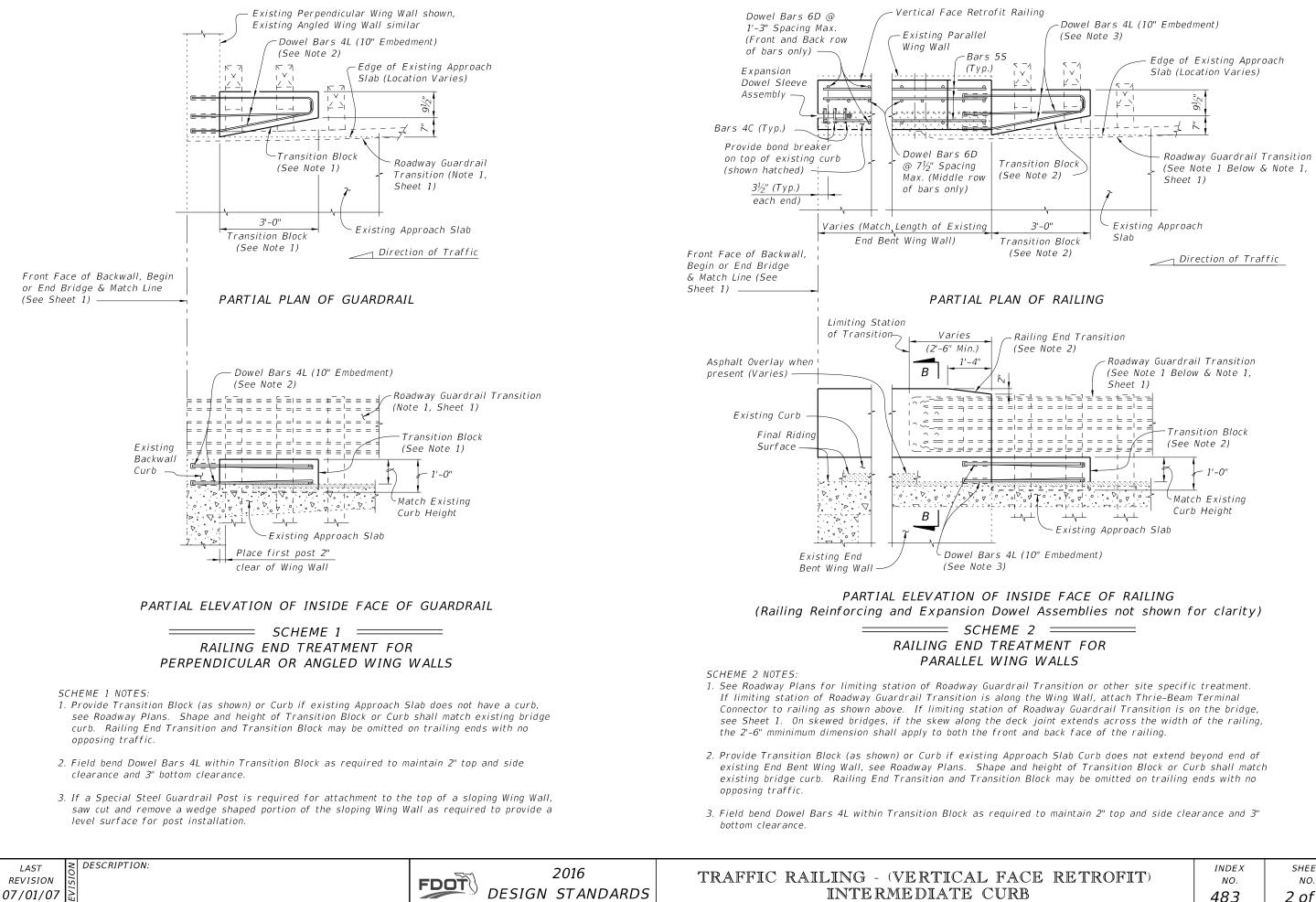


A	
4:37:59	

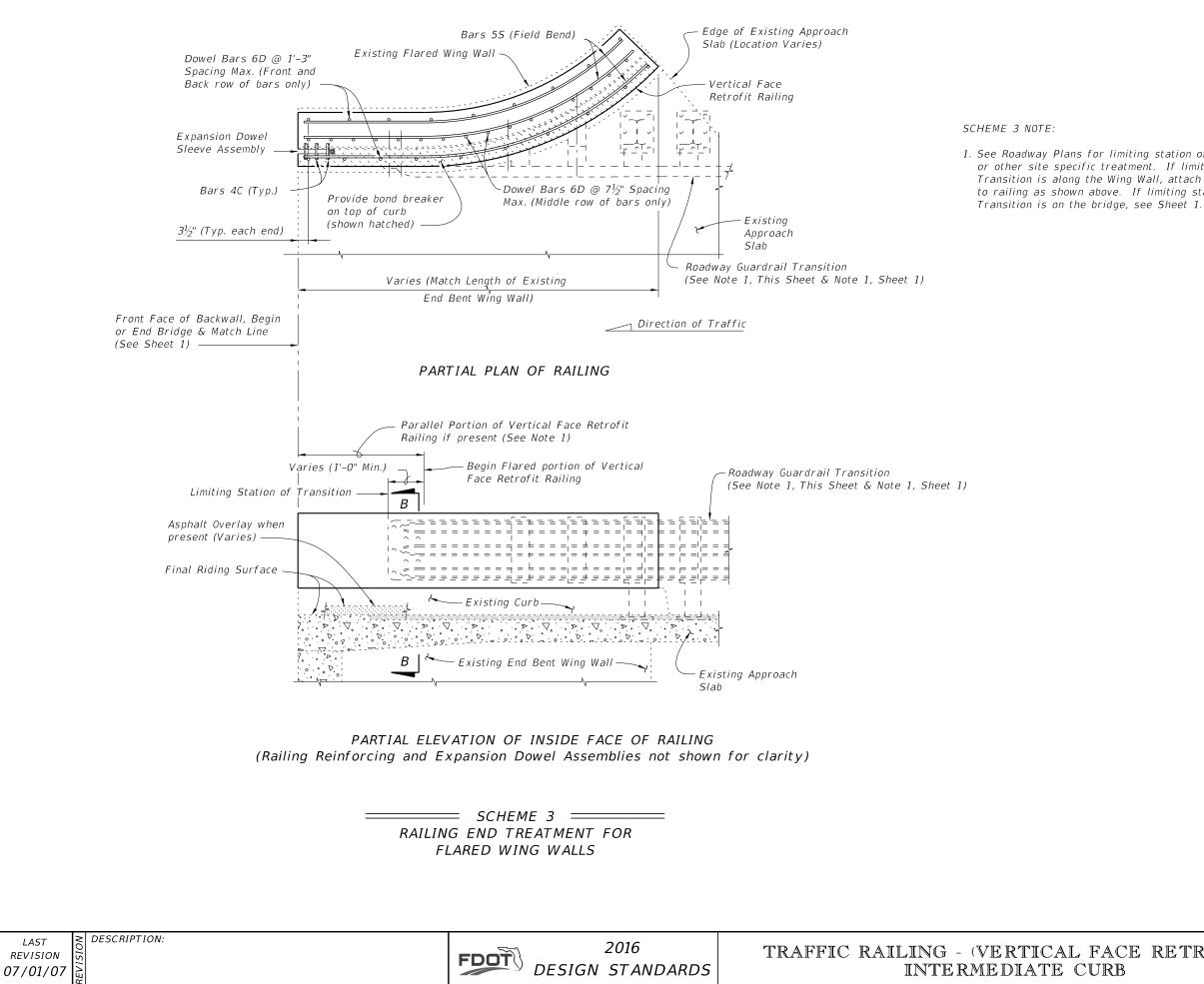






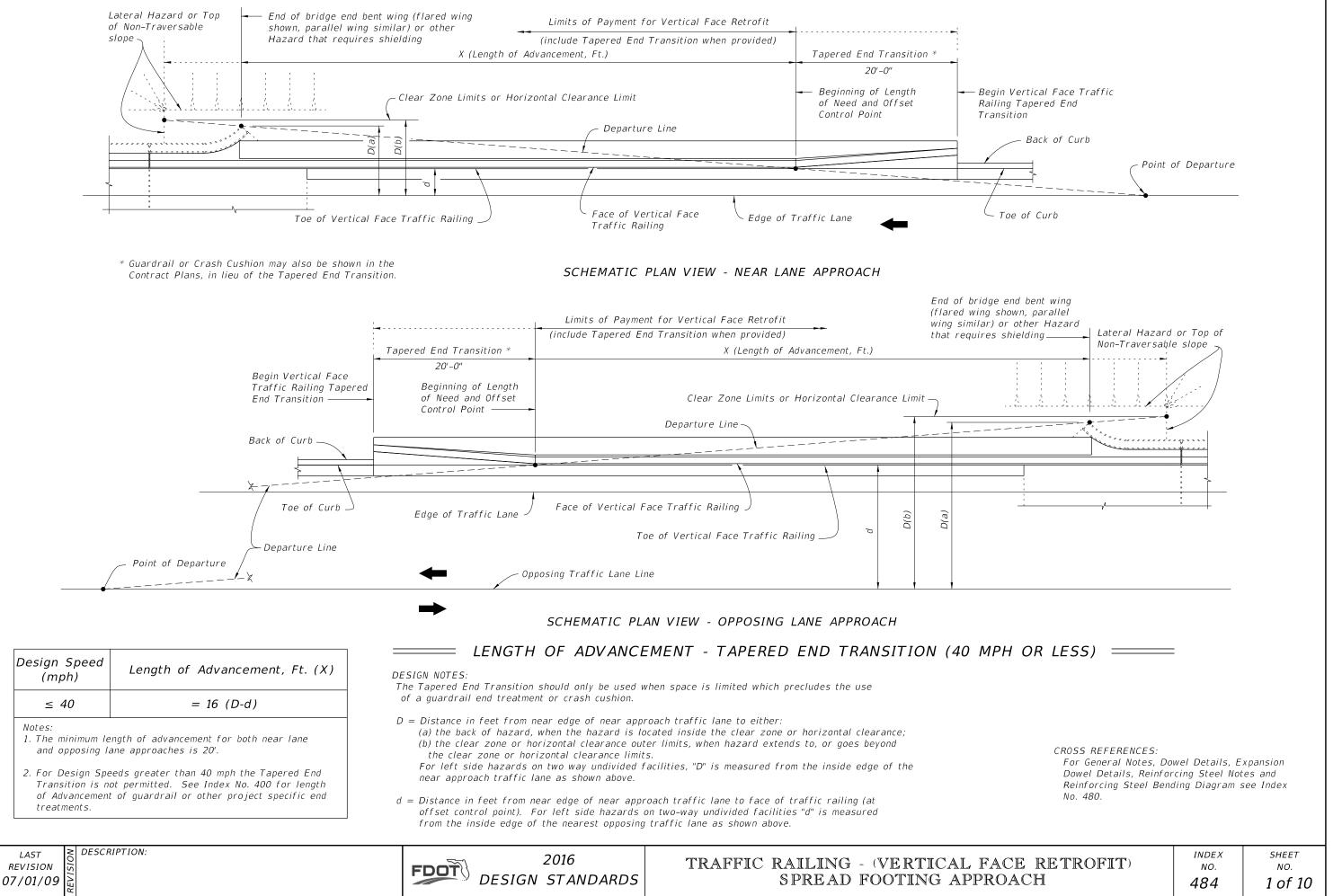


RETROFIT)	INDEX NO.	SHEET NO.
	483	2 of 3

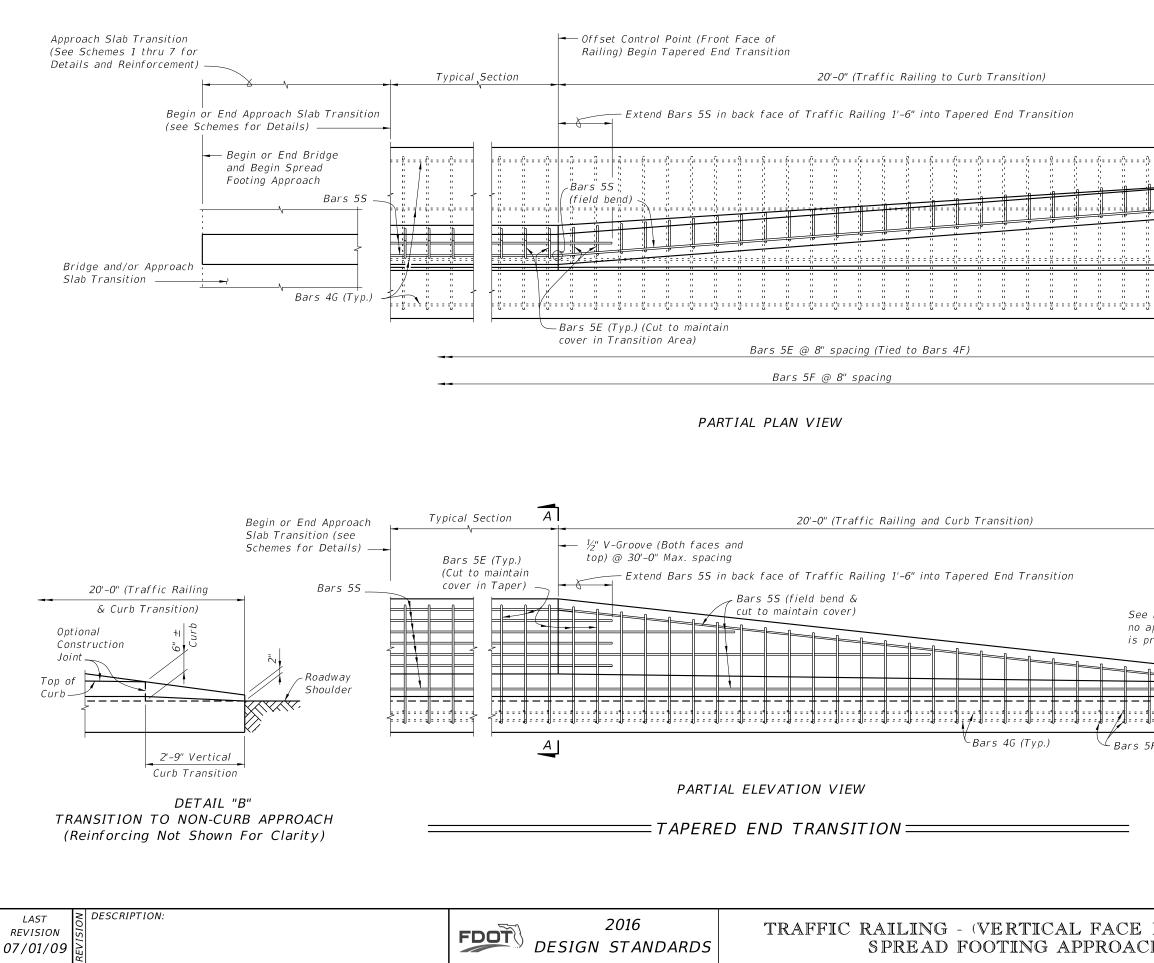


1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail

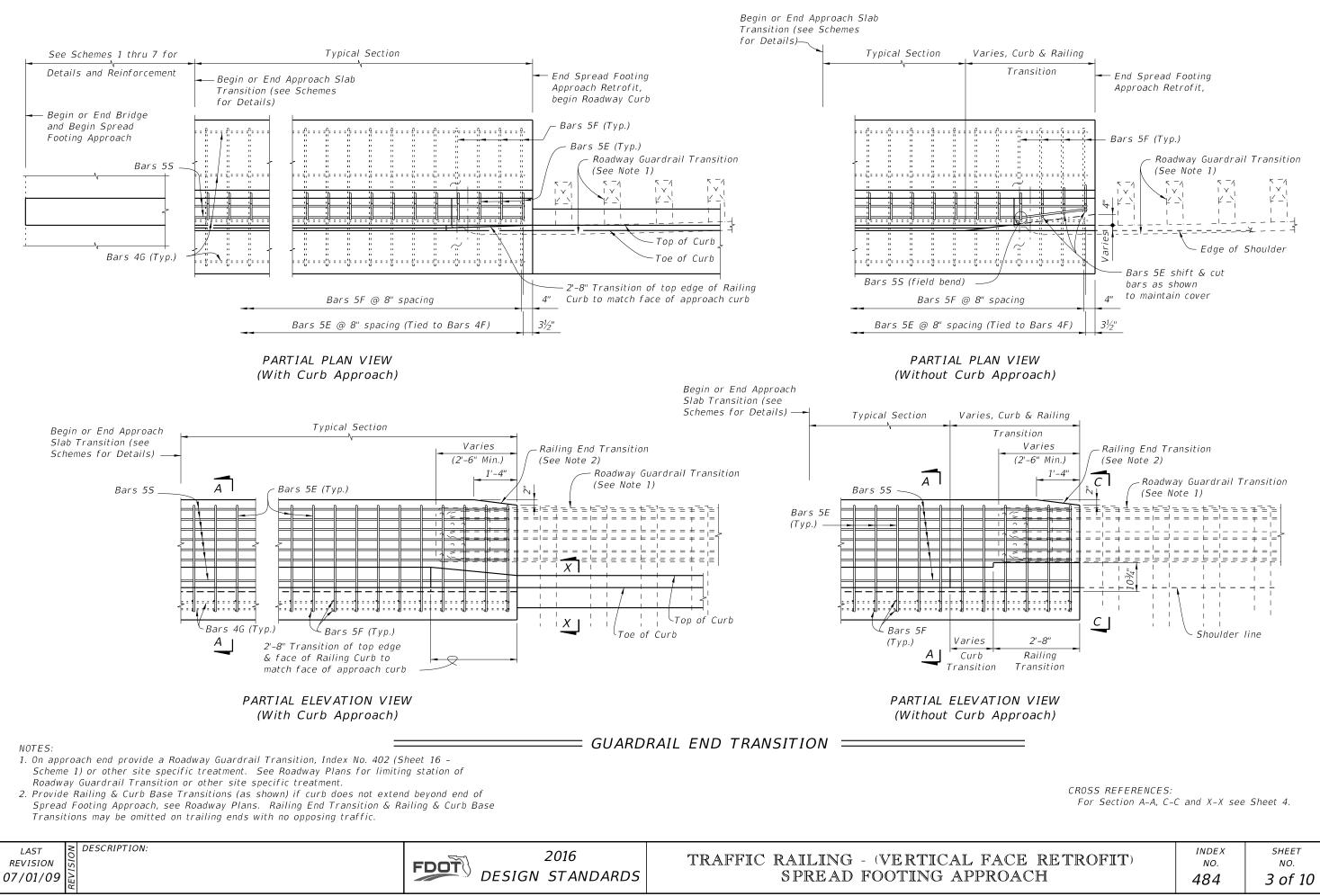
RETROFIT)	INDEX NO.	SHEET NO.
	483	3 of 3



REVISION

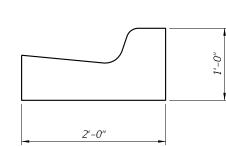


A b	<u>\1</u>	fit, Curb
	Top of Curb	
( RETROFIT) CH	index no. <b>484</b>	sheet NO. 2 of 10

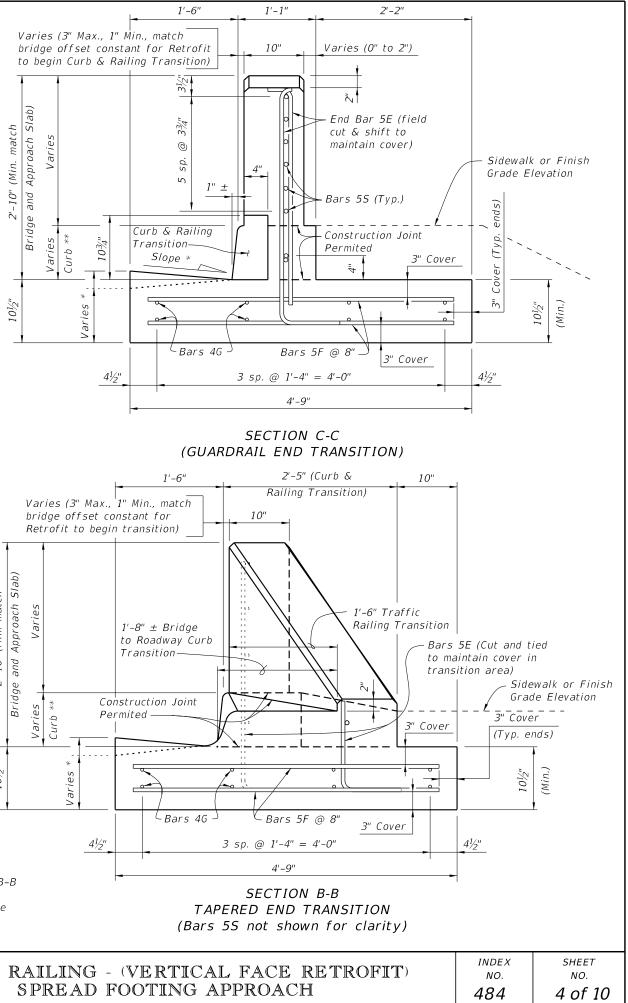


RETROFIT)	INDEX NO.	SHEET NO.
	484	3 of 10

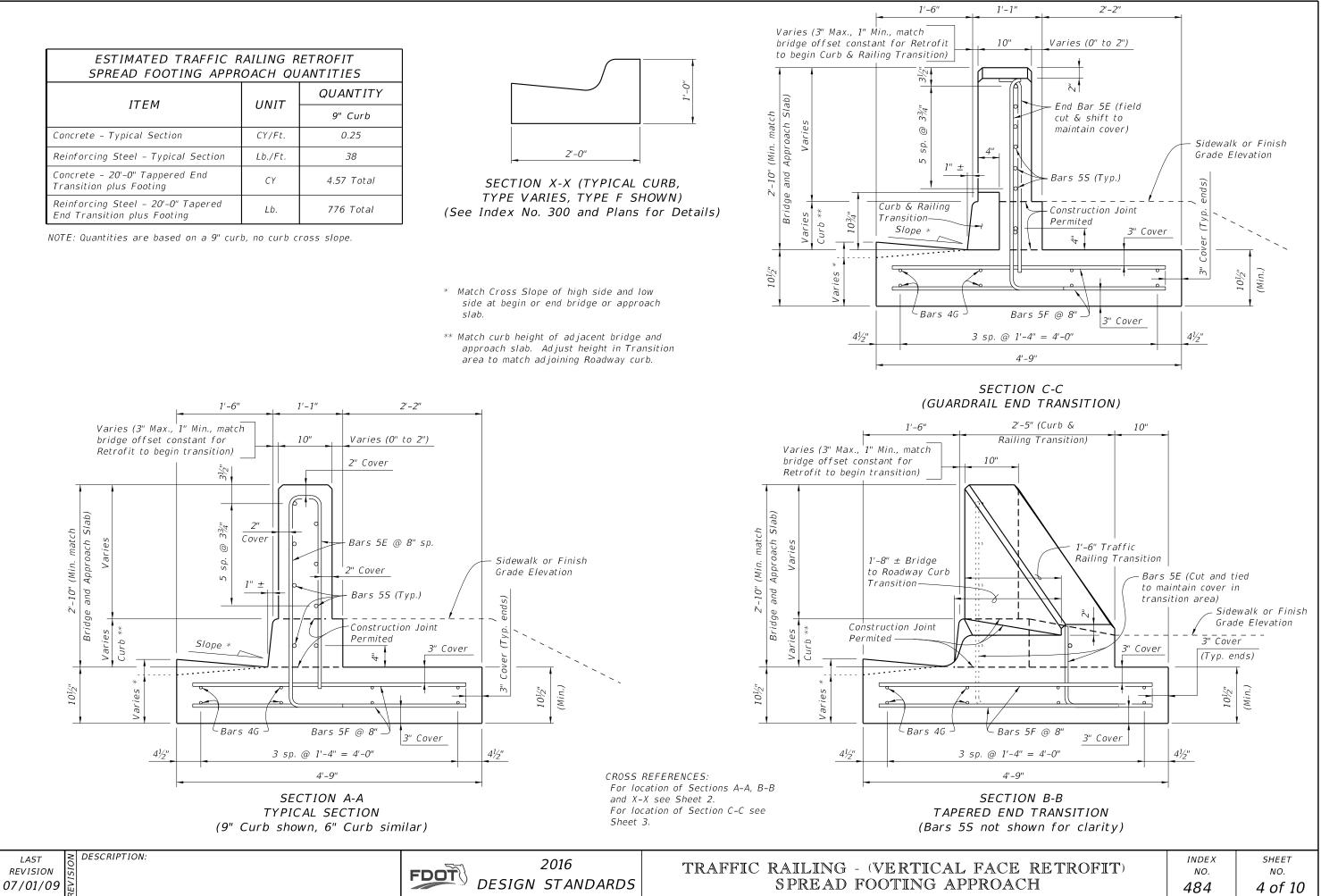
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES				
ITEM	UNIT	QUANTITY		
		9" Curb		
Concrete – Typical Section	CY/Ft.	0.25		
Reinforcing Steel - Typical Section	Lb./Ft.	38		
Concrete - 20'-0" Tappered End Transition plus Footing	СҮ	4.57 Total		
Reinforcing Steel - 20'-0" Tapered End Transition plus Footing	Lb.	776 Total		

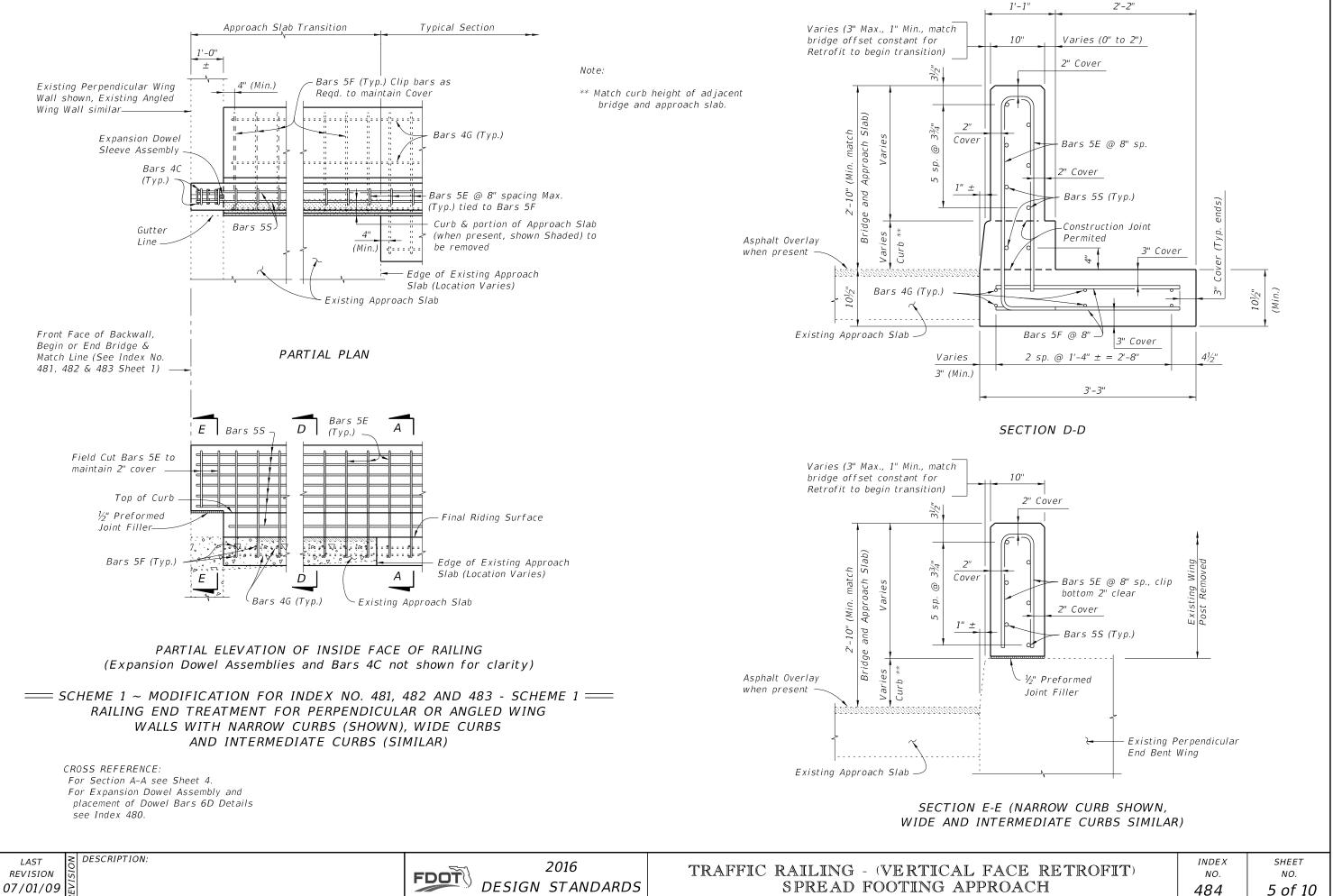


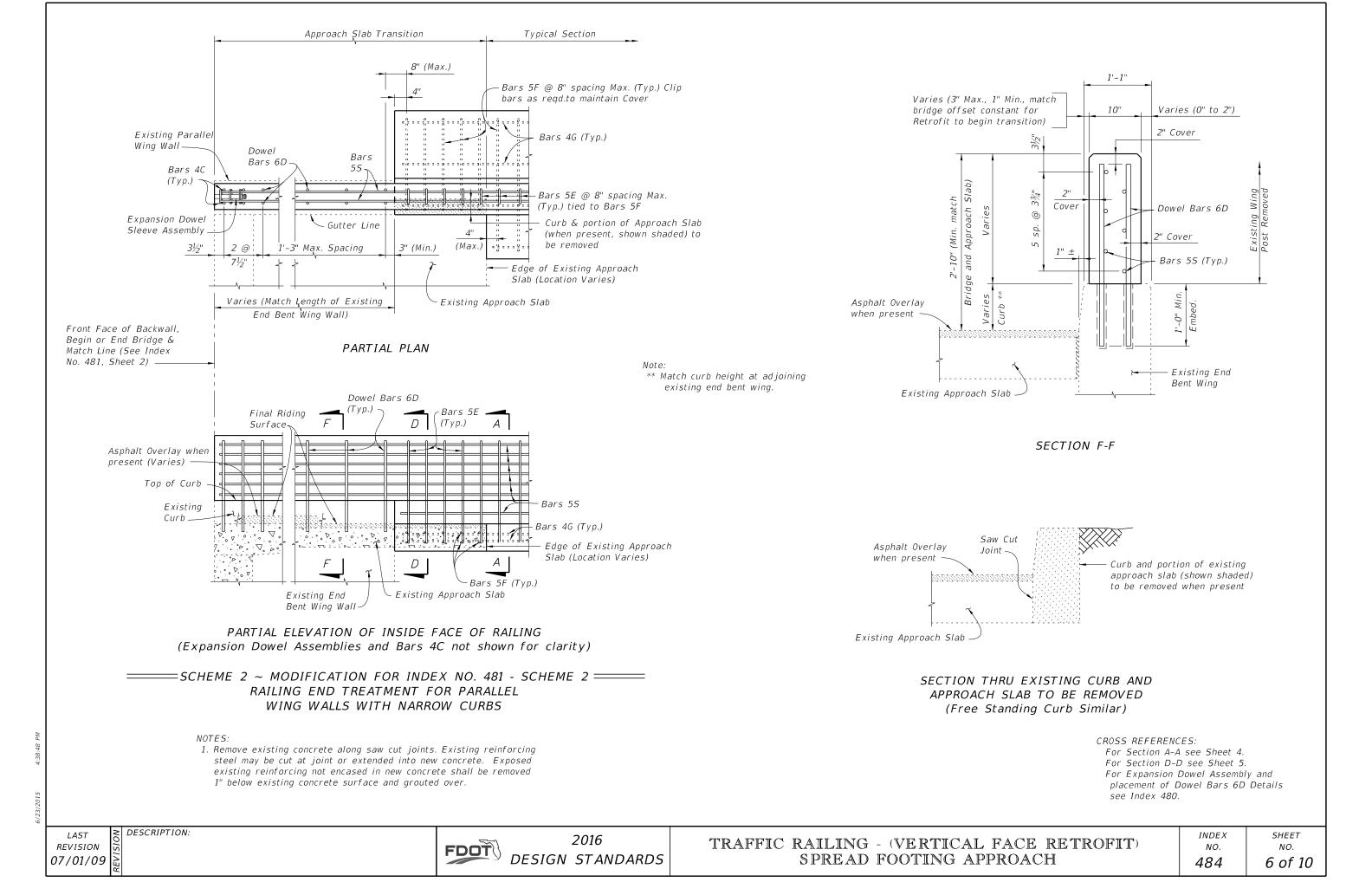
TYPE VARIES, TYPE F SHOWN)

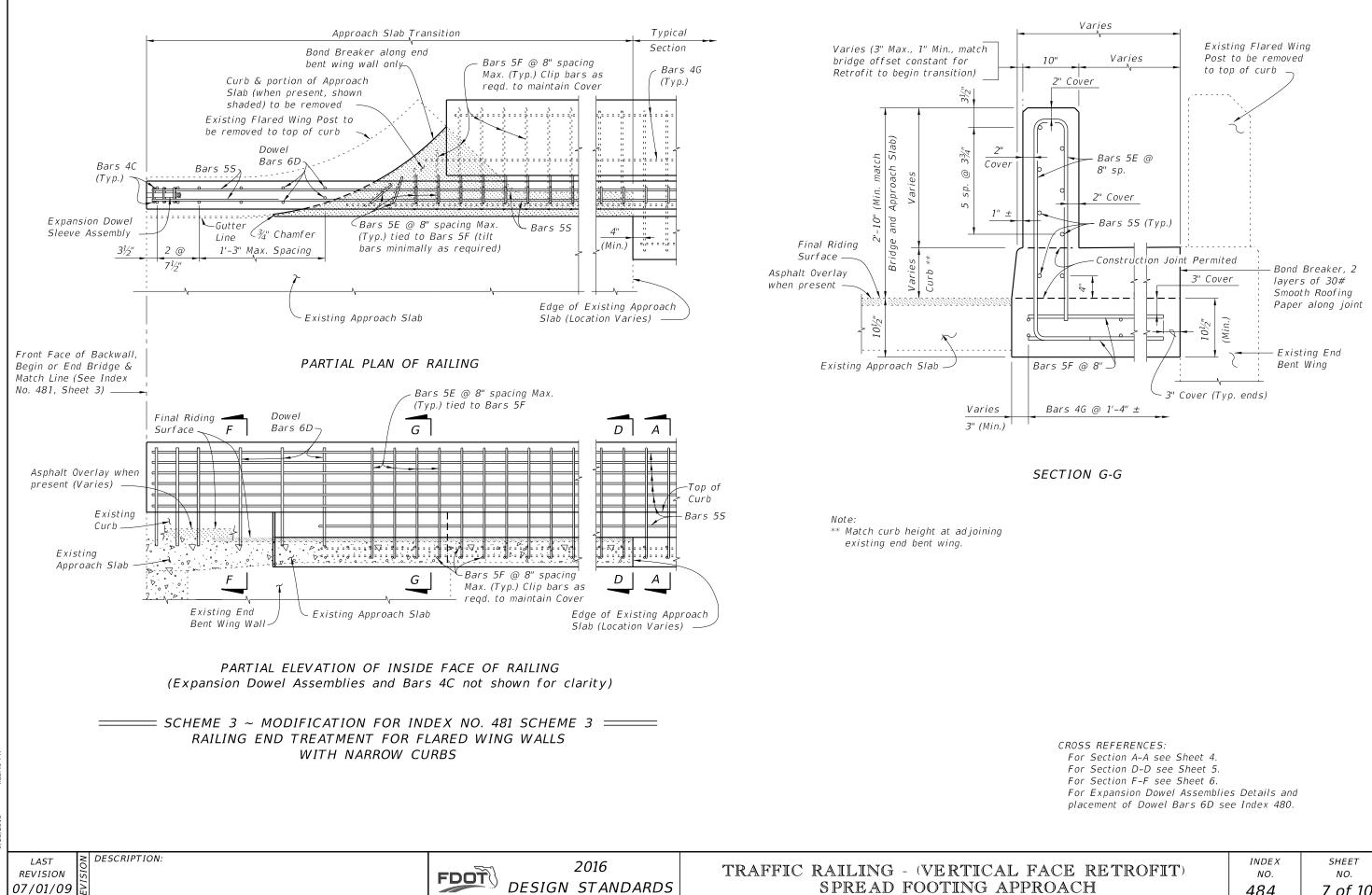


- area to match adjoining Roadway curb.









DSS REFERENCES: For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section F-F see Sheet 6. For Expansion Dowel Assemblie Placement of Dowel Bars 6D se		
CH	index no. <b>484</b>	<sup>sнеет</sup> no. <b>7 of 10</b>

