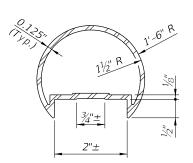


3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

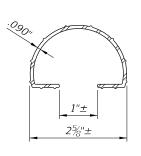
TABLE 1 - RAILING MEMBERS						
MEMBER	ALLOY ⁽¹⁾	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS		
Posts (Type "A" & "B")	6061-T6	RT 2x2x.250	2.00" x 2.00"	0.250"		
Posts (Type "C")	6061-T6	Extrusion $1\frac{1}{2}x2\frac{1}{2}x.125$	1.50" x 2.50"	0.125"		
Top Plate (Type "C")	6061-T6	Extrusion (See Details)	2¾" x 7"	Varies		
To a Doil	COC1 TC	2½" NPS (Sch. 10)	2.875"	0.120"		
Top Rail	6061-T6	3" Round Top Cap Rail	3.000"	0.125"		
Find Harris	6063-T5	2½" NPS (Sch. 10)	2.875"	0.120"		
End Hoops		3.00 OD x 0.125 Wall	3.000"	0.125"		
Tan Bail Jaint/Calina Classes	6063-T5	2.50 OD x 0.125 Wall	2.500"	0.125"		
Top Rail Joint/Splice Sleeves		Top Cap Rail Inner Sleeve	2.800"	0.090"		
Intermediate & Bottom Rail	6061-T6	RT 2x2x.250	2.00" x 2.00"	0.250" ⁽²⁾		
Int. & Bottom Rail Post Connection Sleeve	6063-T5	1.50 OD x 0.125 Wall (3)	1.500"	0.125"		
Handrail Joint/Splice Sleeves	6063-T5	1" NPS (Sch. 40)	1.315"	0.133"		
Handrails	6061-T6	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrail Support Bar	6061-T6	¾" Ø Round Bar	0.750"	N/A		
Pickets (Type 1 Infill Panel)	6061-T6	³¼" Ø Round Bar	0.750"	N/A		
Infill Panel Members (Types 2 - 5)	6063-T5	Varies (See Details)	Varies	Varies		

TABLE 1 NOTES:

- (1) Alloy 6061-T6 or 6063-T52 & T6 may be substituted for Alloy 6063-T5.
- (2) 0.188" wall thickness permitted for rails with post spacings less than 5'-9".
- (3) 1" NPS (Sch. 40) non-slit rail sleeves may be substituted when welded connection Detail "K" is utilized.



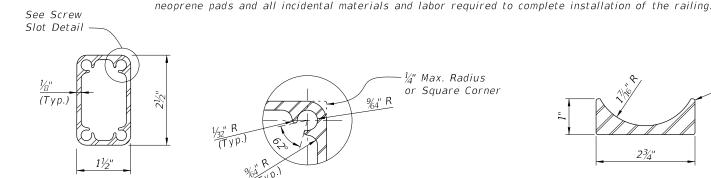
DESCRIPTION:



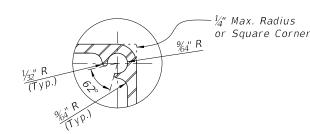
3" ROUND TOP CAP RAIL TOP CAP RAIL INNER SPLICE SLEEVE

ALTERNATE TOP RAIL SECTION ===

ALTERNATIVE BOTTOM & INTERMEDIATE RAIL SECTION FOR TYPE 3, 4 & 5 RAILINGS



POST TYPE "C" SCREW SLOT SECTION



Filler metal for plug welds and bend splices may be ER4043.

shall be galvanized in accordance with Specification Section 962.

DESIGN LOADS, GEOMETRY AND APPLICABILITY:

RAILS, PANELS AND POSTS:

BASE PLATES AND RAIL CAPS:

with the Specifications. RESILIENT AND NEOPRENE PADS:

SHIM PLATES:

ANCHOR BOLTS:

COATINGS:

SHOP DRAWINGS:

SCREW SLOT DETAIL

23/4"

OPTIONAL TOP PLATE EXTRUSION SECTION (POST TYPE "C")

REVISION 07/01/15

2016 DESIGN STANDARDS

ALUMINUM PEDESTRIAN/BICYCLE RAILING

INDEX SHEET NO. NO. 862 1 of 9

NOTES

Adequate foundation support shall be provided for anchorage and stability against overturning (See Sheet 8). See Index No. 861 for special requirements and modifications for use on bridges. The railing shown on these drawings requires a handrail for ramps steeper than a 5% grade to conform with the requirements of the Americans with Disabilities Act (ADA).

Structural Extrusions, Tube, Pipe and Bar shall be in accordance with Table 1 and ASTM B221 or ASTM B429. Top, bottom and intermediate rail corner bends with maximum 4'-0" post spacing, may be Alloy 6063-T6. Perforated panels (Type 5) shall be Alloy 3003-H14. Posts shall be fabricated and installed plumb, ± 1" tolerance when measured at 3'-6" above the foundation. Pickets and vertical panel elements shall be fabricated parallel to the posts, except that Type 2, 3 & 5 panel infills may be fabricated parallel to the longitudinal grade. Corners and changes in tangential longitudinal alignment shall be made continuous with a 9" bend radius or terminate at adjoining sections with mitered end sections when handrails are not required. For changes in tangential longitudinal alignment greater than 45°, posts shall be positioned at a maximum distance of 2'-0" each side of the corner and shall not be located at the corner apex. For curved longitudinal alignments the top and bottom rails and handrails shall be shop bent to match the alignment radius.

Shim Plates shall be aluminum in accordance with ASTM B209, Alloy 6061 or 6063. Shim plates shall be used for foundation height adjustments greater than $\frac{1}{2}$ " between 3 posts and localized irregularities greater than $\frac{1}{2}$ " beneath base plates. Field

Anchor bolts shall be in accordance with ASTM F1554 (Grade 36 for $\frac{7}{8}$ " Ø and Grade 55 for $\frac{7}{6}$ " Ø 4~Bolt Anchorage). Headless anchor bolts for Adhesive Anchors shall be threaded full length. Cutting of reinforcing steel is permitted for drilled hole installation. Expansion Anchors are not permitted. All anchor bolts shall have single self-locking hex nuts. Tack welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only), shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, distort the anchor bolt threads to prevent removal of the nuts. Distorted threads and tack welds shall be coated with a galvanizing compound in accordance

Resilient and Neoprene pads shall be in accordance with Specification Section 932 except that testing of the finished pads

Grind welded joints as necessary to remove burs and weld splatter, additionally remove any sharp edges on rails to prevent injury. Grind all plug welds smooth. Expansion joints shall be spaced at a maximum 40'-0". Field splices similar to the expansion joint detail may be approved by the Engineer to facilitate handling, but top rail must be continuous across

a minimum of two posts. For intermediate and bottom horizontal rails the screwed joints shown on Sheet 4 may be

All welding shall be in accordance with the American Welding Society Structural Welding Code (Aluminum) ANSI/AWS D1.2 (current edition). Filler metal shall be either ER5183, ER5356 or ER5556. Nondestructive testing of welds is not required.

The aluminum railing shall be mill finish unless otherwise noted in the Contract Documents. All nuts, bolts and washers

Details addressing project specific geometry (line & grade) showing post and expansion joint locations, post and panel

type, anchor bolt installation "Case" or lengths, must be submitted by the Contractor for the Engineer's approval prior to

Payment includes rails, posts, pickets, panels, rail splice assembly, base plates, anchor bolts, nuts, washers, resilient or

trim shim plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of trimmed flat shim plates shown. Stacked shim plates must be bonded together with adhesive bonding material and limited to

a maximum total thickness of 1/2", unless longer anchor bolts are provided for the exposed thread length.

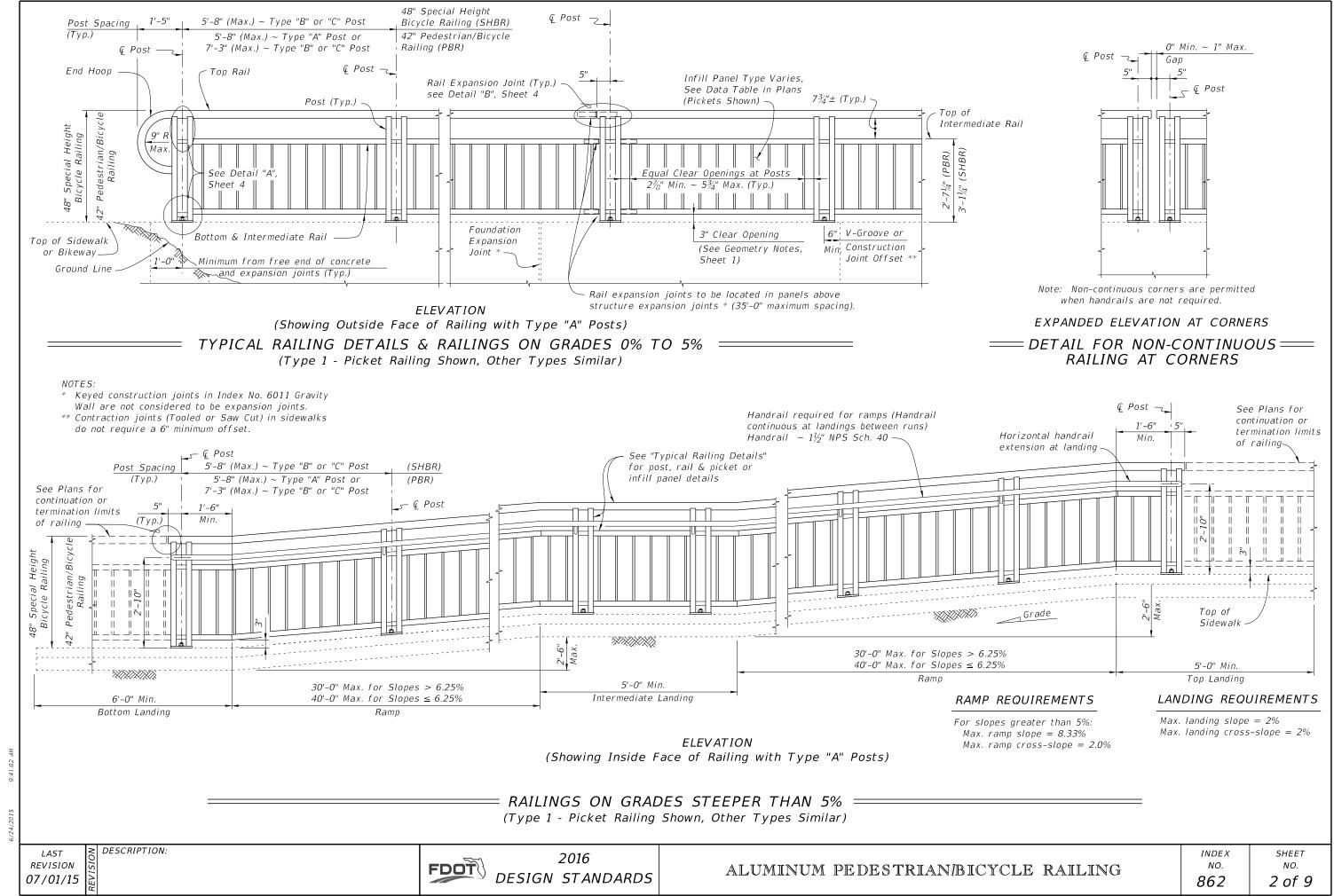
See the Instructions for Design Standards for the design loads, geometry and applicability requirements.

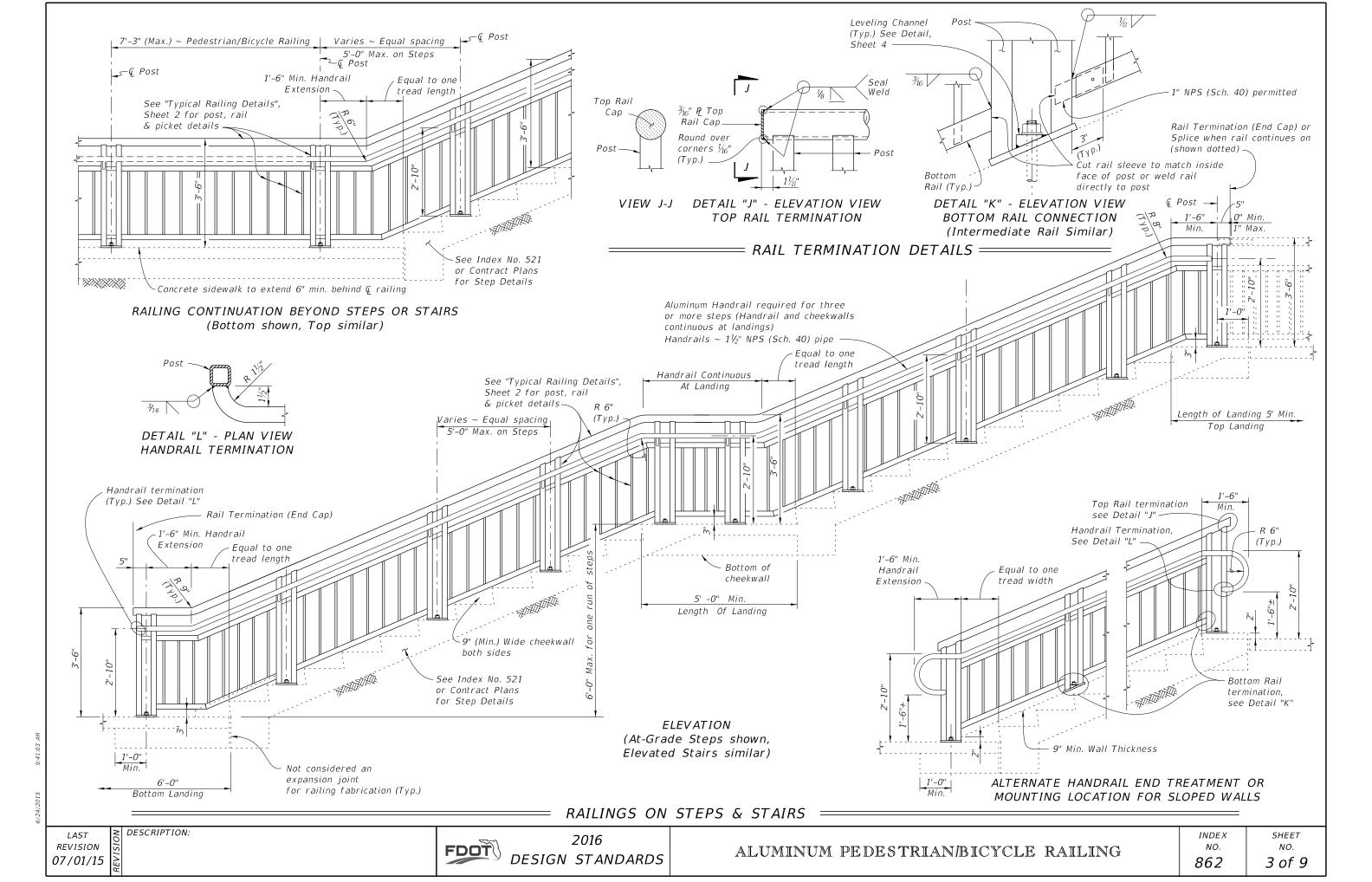
Base Plates and Post Cap plates shall be in accordance with ASTM B209, Alloy 6061-T6.

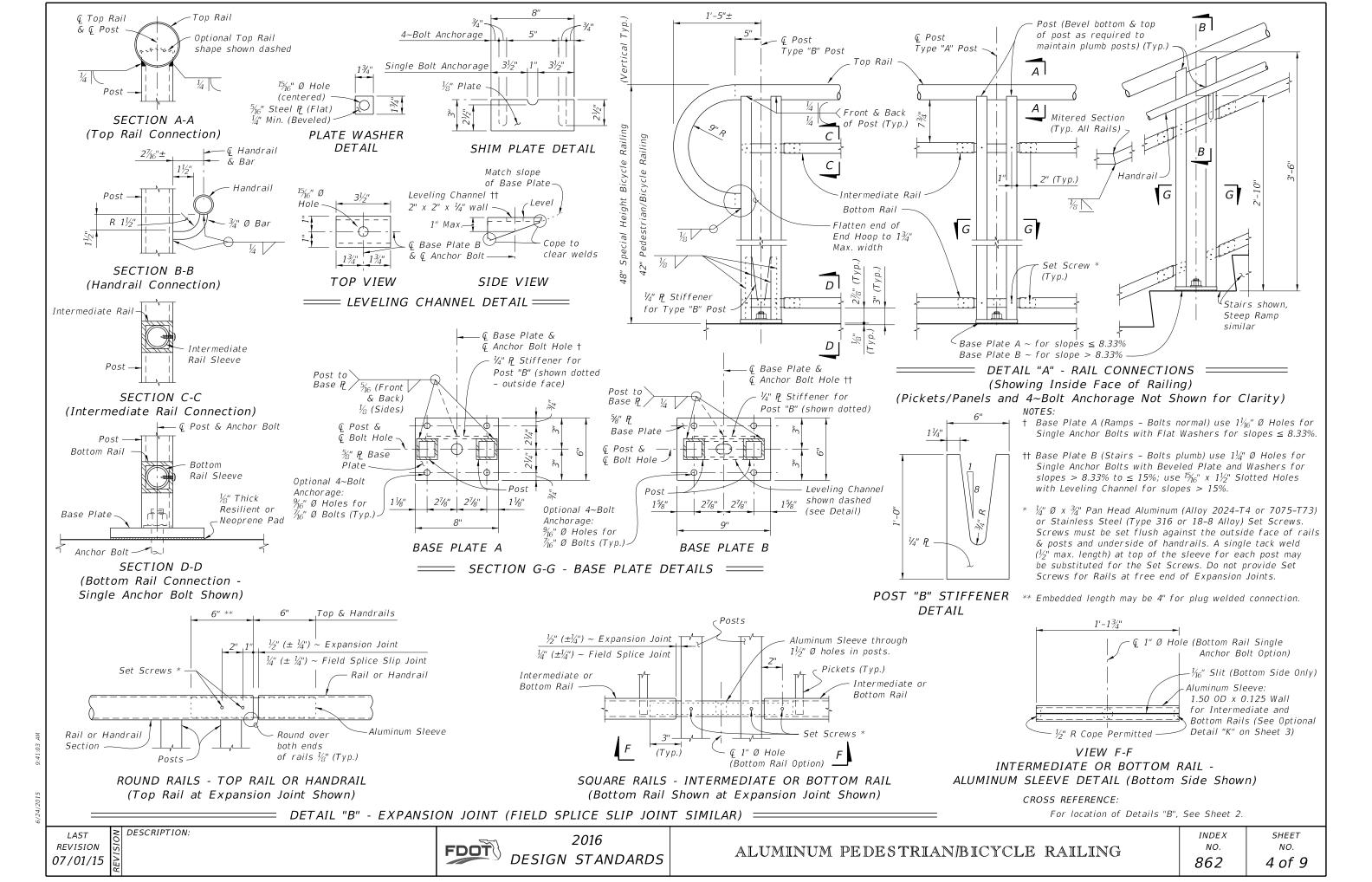
shall not be required. Neoprene pads shall be durometer hardness 60 to 80.

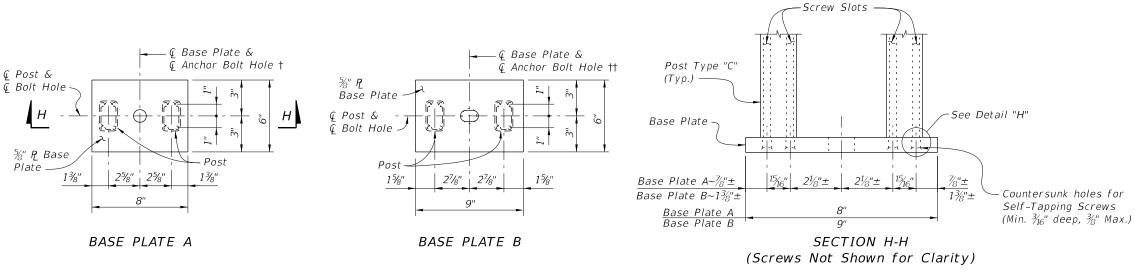
substituted with alternate joints shown on Sheet 3 Detail "K" for Post Type "A" & "B".

fabrication of the railing. Shop drawings shall be in accordance with the Specifications.

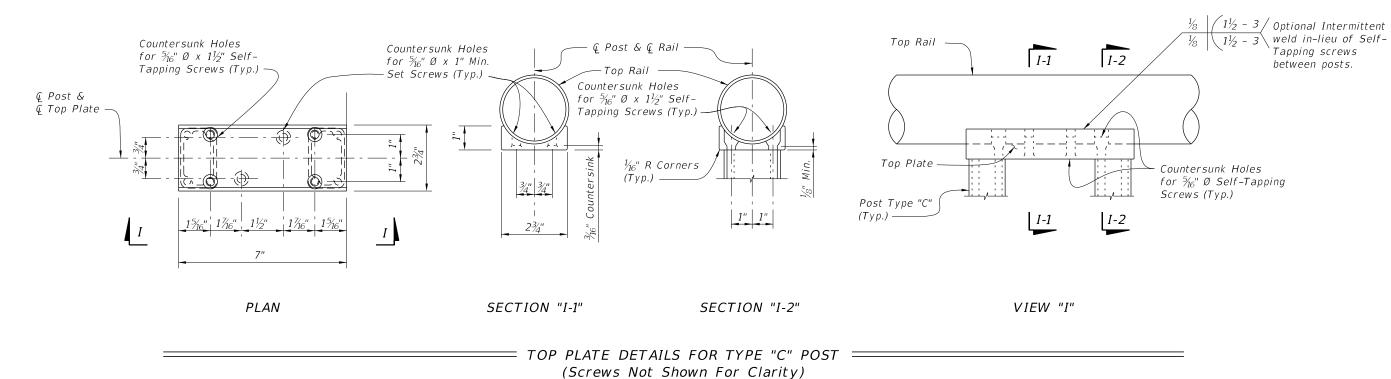












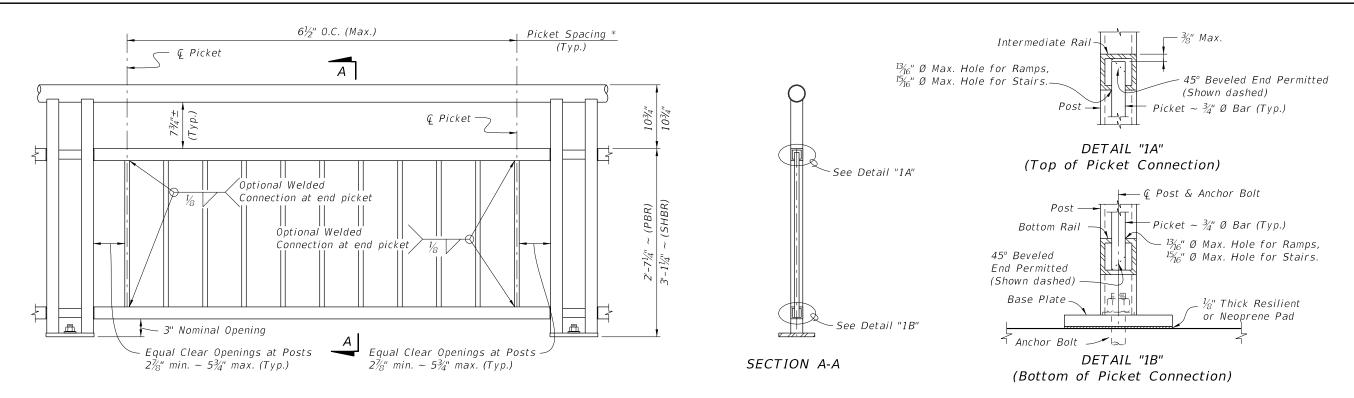
- See Sheet 4 for Notes.
- See Sheet 4 for Notes.
- Length varies for beveled posts on grades. Holes must be drilled plumb to align with screw slot.

DESCRIPTION: 2016 FDOT DESIGN STANDARDS

SHEET

REVISION

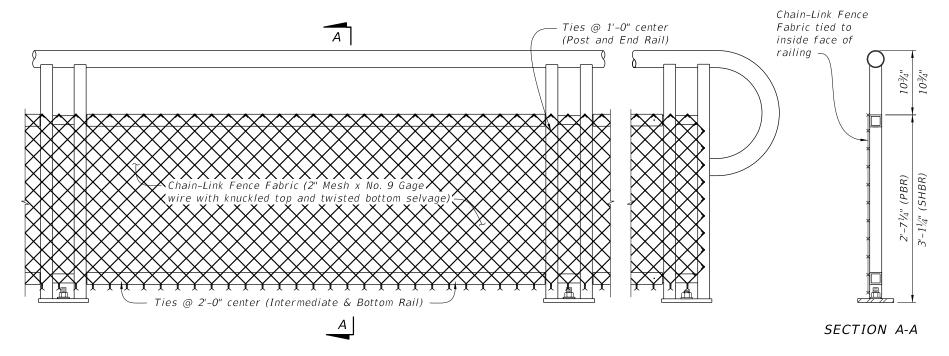
07/01/15



TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. If an alternate design is used, maintain a maximum clear opening of 5% for standard installations and 3% for special conditions.



TYPF	2 -	CHAIN-LINK	(Continuous	Infill	Panel
IIPE	Z -	CHAIN-LINK	(Continuous	1111111	railei)

1. See Plans for Infill Panel option required.

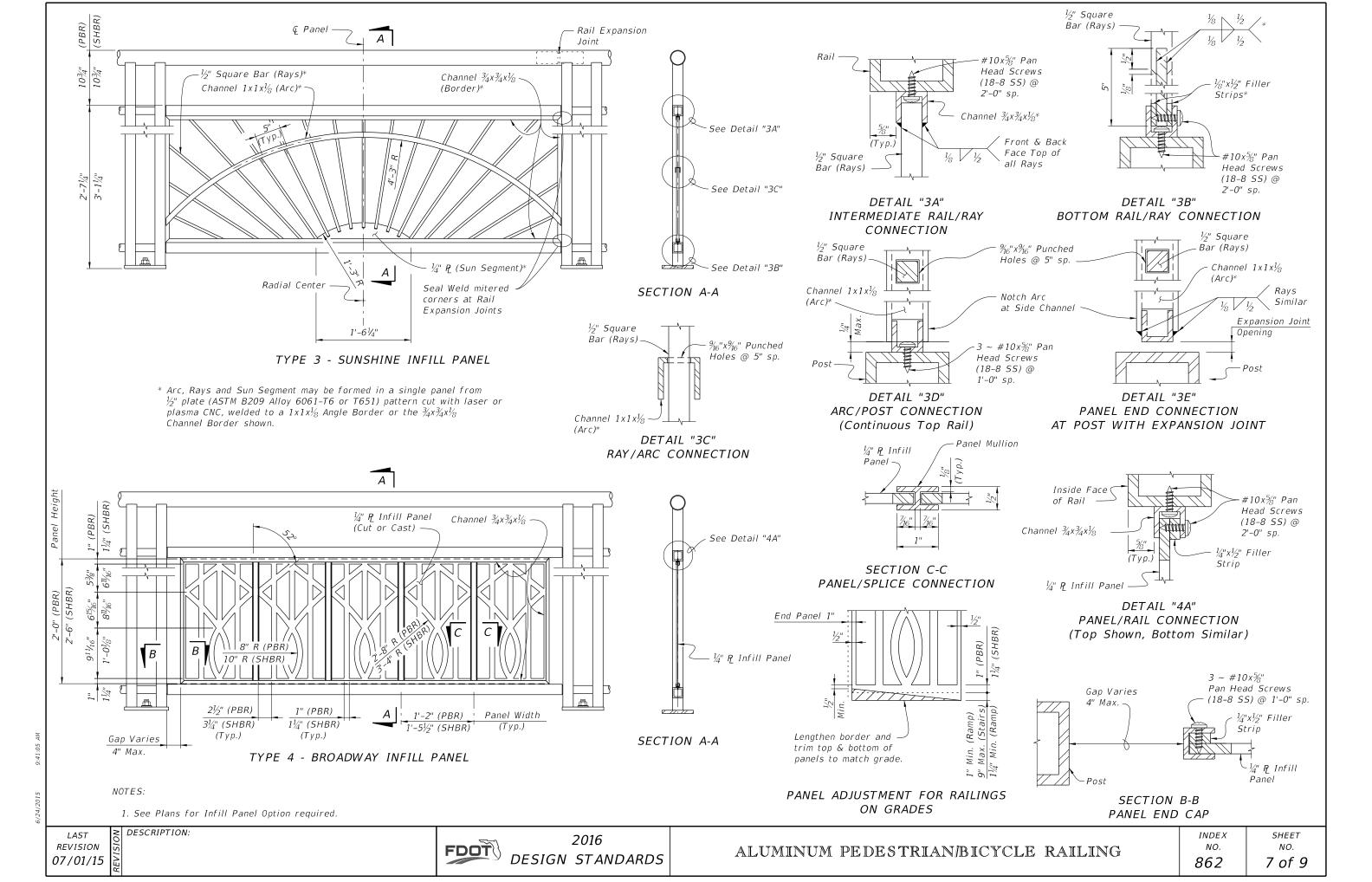
TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS				
COMPONENT	ASTM	COMPONENT INFORMATION		
Chain-Link Fence Fabric (2" mesh with	A 392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating		
twisted bottom and knuckled top selvage)	A 491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)		
	F 668	Polyvinyl Chloride (PVC) Coated Steel - No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.		
Tie Wires	F 626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.		
Tension Bars	F 626	$rac{3}{16}$ " (min. thickness) x $rac{3}{4}$ " (min. width) x 2'-3' (min. height) Steel Bars		
Miscellaneous Fence Components	F 626	Zinc-Coated Steel		

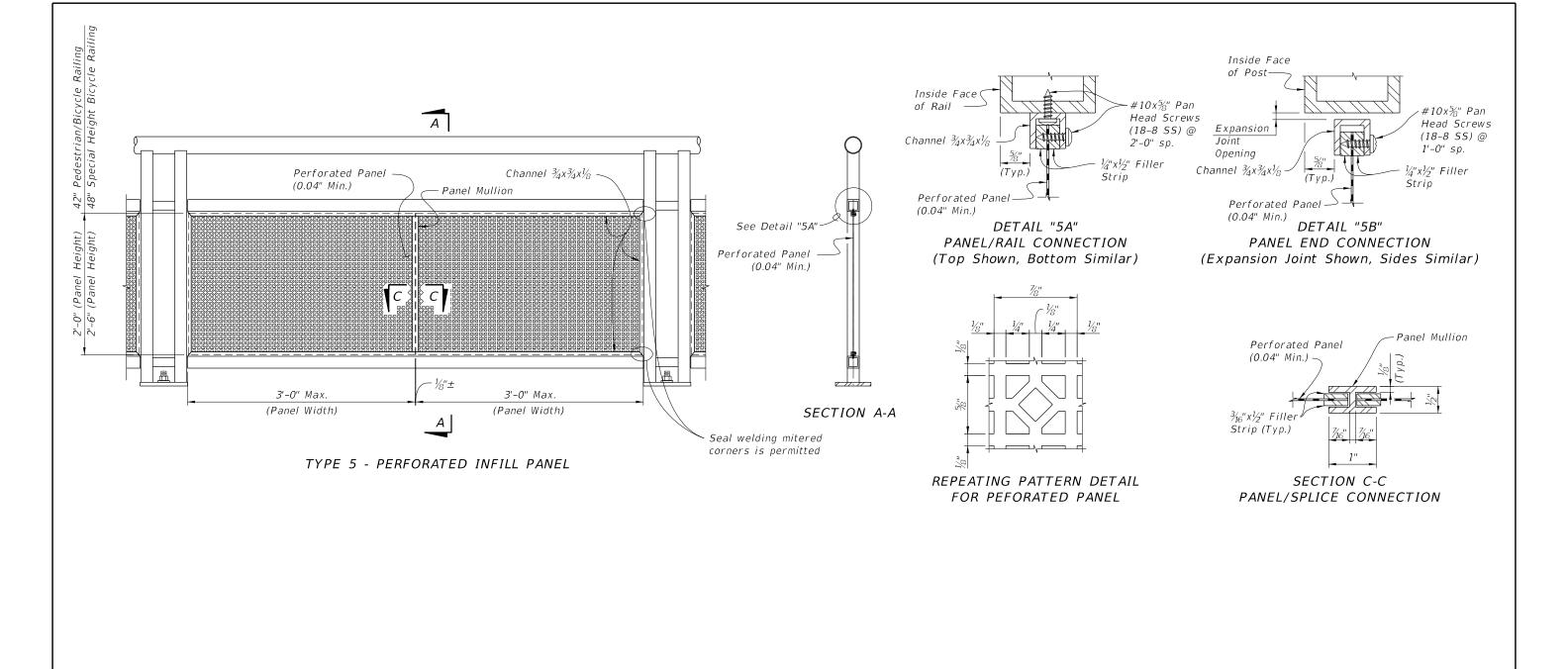
CHAIN-LINK PANEL NOTE:

Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

DESCRIPTION: **REVISION** 07/01/15

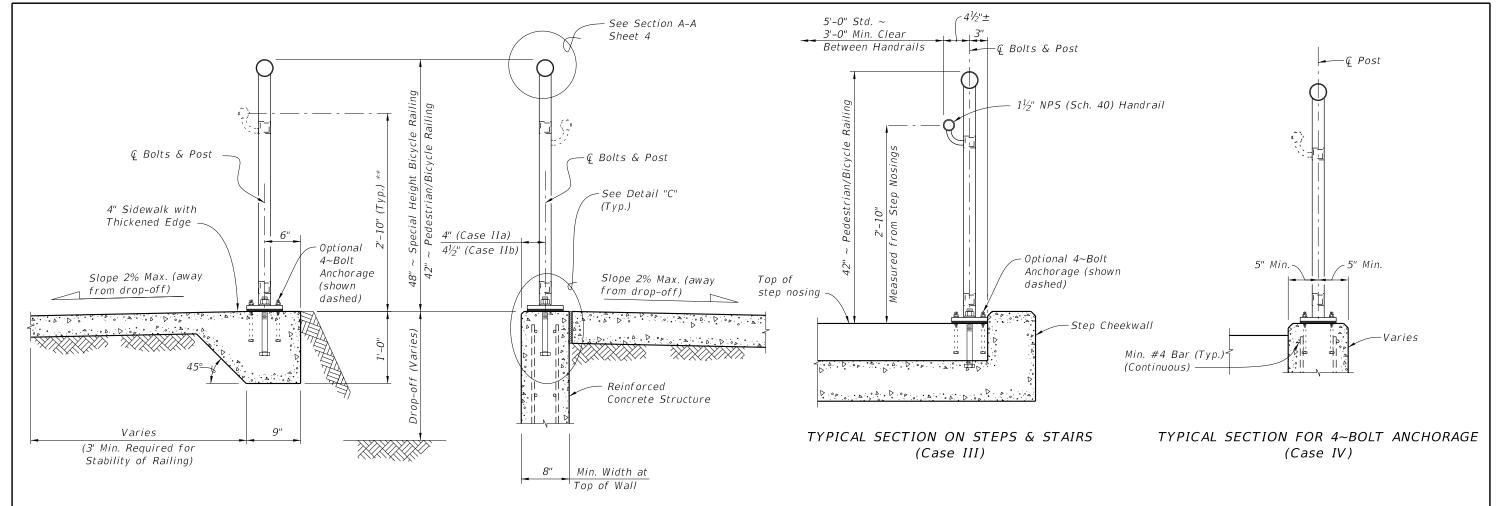






DESCRIPTION: REVISION 07/01/15

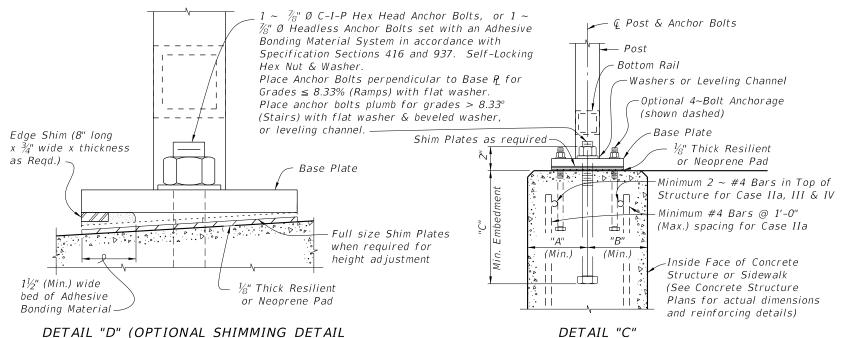
FDOT



TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

TYPICAL SECTION ON RETAINING WALL (Case II)



DETAIL "C"					
(Cast-In-Place	Anchor	Bolts	shown,		
Adhesive	Anchors	simil	ar)		

	ANCHOR BOLT TABLE						
CASE STRUCTURE TYPE	DIMENSIONS			ANCHOR LENGTH		ANGUOR	
	"A" Edge Dist.	"B" Edge Dist.	"C" Embedment	C.I.P Hex Head Bolt	Adhesive Anchor	ANCHOR SIZE	
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	½" Ø
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø
IIb	Gravity Wall Index No. 6011	4½"	3½" @ top	1'-0" *	1'-1½"	1'-2"	½" Ø
III	Step Cheekwall	4 ¹ / ₂ "	4½"	9"	10½"	11"	%" Ø
IV	Varies	5"	5"	5"	6½"	7"	7₁6" Ø

^{*} Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".

REVISION 07/01/15

2016 DESIGN STANDARDS

ALUMINUM PEDESTRIAN/BICYCLE RAILING

INDEX SHEET NO. 862

NO. 9 of 9

^{**} When required; measured from top of sidewalk (Typ.)