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**ALTERNATIVE VALIDATION PRACTICE
OF AN AUTOMATED FAULTING MEASUREMENT METHOD**

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ABSTRACT

A number of states have adopted profiler based systems to automatically measure faulting, in jointed concrete pavements. However, little published work exists which documents the validation process used for such automated faulting systems. This paper documents an alternative practice for making an initial assessment of a newly developed automated faulting method. Findings from this experiment show that a high speed inertial profiler used in conjunction with a faulting reference device provides a practical validation method under controlled conditions. Furthermore, the algorithm which controls the automated faulting measurement method provides reliable, highly repeatable and reproducible faulting results. This paper also documents the test equipment used in the experiment as well as the data collection efforts, the data analysis and subsequent findings and recommendations.

89 INTRODUCTION

90 The AASHTO provisional standard for joint faulting measurement defines faulting as the
91 elevation between two points of measurement (P1 and P2) to the nearest 1mm (0.04 in.), with a
92 difference of 5 mm (0.2 in.) defined as the threshold for faulting [1]. This standard is applicable
93 to both manual as well as automated methods. In the past, automated joint faulting measurement
94 was an area that did not receive great emphasis by many agencies. However, the new Highway
95 Performance Monitoring System (HPMS) reassessment model requires state highway agencies to
96 collect network-level faulting data in accordance with the AAHTO R36-04 protocol, which is
97 intended to measure faulting with a vehicle at highway speed [2]. This requirement in addition to
98 the disadvantages associated with manual data collection methods created a renewed interest in
99 automated technologies that collect data at highway speeds. A number of states have adopted
100 inertial high speed profiler based systems to automatically collect faulting, smoothness, rut depth
101 and other pavement characteristics. However, there is little published work regarding the
102 validation of automated faulting measurement systems.

103

104 GOAL AND OBJECTIVE

105 The objective of this study is to evaluate the accuracy and precision of a HSIP based
106 automated fault measurement system using a two-phase approach. The first phase approach
107 evaluates the high speed inertial profiler's (HSIP) ability to produce reliable faulting
108 measurements under controlled conditions. The second phase tests the validity of the automated
109 method to produce repeatable and reproducible results under normal field conditions. The goal is
110 to use the results from this study to support the implementation of the automated fault
111 measurement system into FDOT's Annual Pavement Condition Survey (PCS) process [3].

112

113

114 SCOPE

115 A two- phase approach was used to validate the HSIP and the automated faulting method.
116 In the first phase, automated faulting measurements were performed at various speeds using a
117 single HSIP. This approach was selected to test the ability of a HSIP to measure faulting under
118 controlled conditions by virtually eliminating the effects of surface texture and vehicle wander.
119 For this purpose, an aluminum device was manufactured to serve as a reference or ground truth.
120 The device consists of seven C-channel extrusions secured to a support plate, which simulate
121 jointed concrete slabs with different faulting magnitudes. In the second phase of the experiment a
122 rigid pavement section was used to validate the automated faulting method under normal field
123 conditions. Five HSIP operated by different operators, and a manual Faultmeter were used in this
124 phase of the study.

125

126 EQUIPMENT

127

128 *Georgia Faultmeter*

129 This hand operated device weighs approximately 7 lbs (3.2 kg) and supplies a digital
130 readout with the push of a button located on the carrying handle (Figure 1). The readouts are
131 displayed to the nearest 0.1 mm with a positive or a negative sign, representing positive faulting or
132 negative faulting, respectively. The Faultmeter's support feet are positioned on the leave side of
133 the slab joint, pointing in the direction of traffic while the measuring probe is in contact with the
134 approach side of the slab. The joint is centered between the guide marks visible on the side of the

135 meter. The vertical movement of the probe is transmitted to a Linear Variable Displacement
136 Transducer (LVDT) to measure joint faulting. A slab which is lower on the leave side of the joint
137 will register a positive faulting value. If the slab leaving the joint is higher, the meter gives a
138 negative reading [4].
139

140 ***High Speed Inertial Profiler (HSIP)***

141 Five HSIP vehicles operated by five different operators were used in the second phase of
142 this project. The HSIP consisted of a full-size van equipped with various electronic sensors
143 (Figure 2). Three laser height sensors laser sensor were mounted in the front of a specially
144 designed bumper of each host vehicle. Two 32 KHz Selcom 5000 laser sensors to measure
145 longitudinal profiles, and a 16 Khz laser mounted in the middle of the bumper which primarily
146 used for rut depth measurement. The typical single spot height laser footprint measured 0.13 in
147 (3mm) in diameter. The HSIP is also equipped with accelerometers mounted in tandem with each
148 wheel-path height sensor to compensate for the vertical motion of the vehicle body [5]. The HSIP
149 were also equipped with data acquisition systems to collect and store elevation profile data of the
150 traveled surface. One of the HSIP, known as the Multi-Purpose Survey Vehicle (MPSV), was
151 equipped with a forward-view camera, an INO Laser Road Imaging System (LRIS), a Laser Rut
152 Measurement System (LRMS), and a Differential Global Positioning System enabled Position and
153 Orientation System (POS). The LRIS system is comprised of two high-resolution line-scan
154 cameras and two high-power laser line projectors aligned in the same plane, and configured to
155 image almost 13 ft (4 m) wide pavement sections with a 0.04 in. (1mm) resolution at speeds that
156 can surpass 62 mph (100 km/hr).
157

158 ***Faulting Reference Device***

159 The device consists of a 0.24 in x 48 in aluminum base plate, which supports seven 0.25 in
160 x 8 in x 6 in C-channel extrusions ranging from 0.036 in (0.96 mm) to 1.96 in (49.92 mm) in
161 height (Figure 3). The different in height between any two adjacent C-channel extrusions are used
162 as reference measurements to simulate faulting. Different “joint” widths can be obtained by
163 adjusting the spacing between the C-channels after being moved along longitudinal grooves cut
164 into longitudinally on the upper side of the base plate. An Allen wrench was used to lock the
165 channel extrusions in place after a 6.4 mm joint spacing was obtained. Multiple measurements of
166 each C-channel extrusion height were performed with a Starrett No. 721A Electronic Digital
167 Caliper calibrated calipers, rated at 0.0005 in. (0.01 mm) resolution and a ± 0.001 in. (± 0.03 mm)
168 accuracy. The difference in height between adjacent C-channels were calculated and recorded and
169 were later used as reference to compare with the HSIP measurements.
170

171 ***Automated Faulting Program***

172 The automated faulting program used in this experiment is an enhancement to the earlier
173 version of the program [6]. The following steps describe the process used by the current program
174 to identify transverse joint locations and to calculate faulting magnitude:
175

- 176 1. The program checks the elevation points along a given profile and removes user-defined
177 exclusion areas such as bridges.
- 178 2. The program sets a default sensitivity factor (SF) equal to 0.5 x the HSIP sampling rate,
179 which represents the minimum slope between any two consecutive points along the
180 profile.

- 181 3. The program looks for valleys (i.e. negative slope) and peaks (i.e. positive slope) along the
 182 profile which meet the minimum slope criterion described in step 2.
 183 4. The program then calculates the distance between the identified peaks and valleys. If the
 184 distance is less than 2.5 inches, the program considers a valley to be the location of a joint.
 185 The 2.5 inch spacing was selected to ensure at least two elevation points are captured
 186 within a joint when using a sampling interval less than 1 inch.
 187 5. Faulting is then calculated based on the AASHTO R36-04 criteria.
 188 6. The program checks whether the computed faulting is greater than $1/64^{\text{th}}$ inch; this is based
 189 on the FDOT PCS specification which only considers faulting greater than $1/32$ inch [3].
 190 7. If the faulting is greater than $1/64$ inch, the corresponding joint location is temporarily
 191 saved into a joint location array.
 192 8. The program repeats steps 3 through 7 for all points along the given profile.
 193 9. The program looks up the joint location array to check the distance between any two
 194 consecutive joints is less than 14.8 inch. This is to adhere to the AASHTO criteria which
 195 require faulting be calculated using elevation points between 3.0 and 8.8 inch away from a
 196 joint. It is also to ensure that the elevation point(s) within 3 inches of a joint are not used
 197 to calculate faulting at any adjacent joint. If two joint locations are less than 14.8 inches
 198 apart, the program will only keep the one with the deepest fault.
 199 10. The program counts the number of joints in the array and then clears the array.
 200 11. The program goes through four more iterations repeating steps 2 through 10 using a binary
 201 search each time changing the SF. If the program does not find a number of joints greater
 202 than previously found, it uses the number of joints detected in the previous iteration.
 203 Otherwise, it continues the process until it cannot find a larger number of joints and keeps
 204 the last SF.
 205 12. The program recalculates all the joint locations and magnitudes using the best SF which
 206 yields the largest number of joints as determined in step 11 and saves this information into
 207 the joint array.
 208

209 DATA COLLECTION

210 *Phase 1 - Simulated Faulting*

212 This phase of the validation process was to test the HSIP under controlled conditions of
 213 variables typically encountered during a typical profile survey such as pavement texture and
 214 vehicle lateral wander. This methodology is a practical and a relatively safe method to make a
 215 quick assessment of the HSIP's ability to collect accurate and repeatable elevation points at
 216 highway speed. The Gainesville Speedway racetrack was used to conduct this part of the
 217 experiment. The HSIP's infrared target sensor mounted in the middle of the vehicle's front
 218 bumper was adjusted to ensure proper alignment with the faulting reference device. Reflective
 219 tape was placed at both ends of the device to trigger the HSIP data acquisition system. An initial
 220 run was conducted to test the system's operability, to check for the alignment of the middle laser
 221 sensor with the centerline of the reference device, and to check for any obstructive artifacts on the
 222 pavement surface. Three replicate passes were conducted by each HSIP at operating speeds of 50,
 223 60 and 70 mph which had slightly different smallest sampling intervals ranging from 0.681in (17.3
 224 mm) to 0.910 in (23.1mm). Since the HSIP middle lasers were not equipped with an
 225 accelerometer, the corresponding profile elevations were corrected using the average readings
 226 from the left and right accelerometers. All profiles were processed through the vendor software to

227 generate profile elevation files after a 300 ft wavelength filter was applied. The output files were
 228 saved as .csv files which were then imported into Microsoft Excel for analysis (Figure 4).
 229
 230

231 ***Phase 2 - Field Validation***

232 State Road (SR) 331 is a two-lane joint plain concrete pavement (JPCP) and was selected
 233 for its proximity to the FDOT State Materials & Research Office, the relatively low vehicular
 234 traffic volume and operating speed, and the relative ease for setting up traffic control. Most rigid
 235 pavements in Florida are located on limited access facilities and the logistics involved in
 236 conducting a comprehensive field validation operation requires substantial staff and equipment
 237 resources, in addition to the potential impact such an operation could have on the safety of project
 238 staff and the traveling public. This can add a significant demand on any agency's budget
 239 especially when operating with limited resources is the opus operandi.
 240

241 The southbound inside test lane of SR 331 was closed to traffic during the entire data
 242 collection operation which took place in the middle to late afternoon for two consecutive days.
 243 This time window was selected to minimize the effects of slab curling and warping. The 2,000 ft
 244 (609.6m) test section included a 500 ft (152.4m) lead-in and lead-out, and a 1,000 ft (304.8m)
 245 effective test length spanning over 49 concrete slabs. The slabs were 20 ft (6.1m) long by 12 ft
 246 (3.7m) wide with a relatively smooth surface finish. Spray paint was applied at nine locations
 247 spaced two inches apart across the right wheel-path. Nine faultmeter measurements were taken at
 248 these marked locations along each leave slab joint which was identified by a sequential number
 249 painted on the approach side of the slab. The four similar HSIP performed three repeat passes
 250 each while the MPSV performed five repeat passes, all vehicles operated at a maximum posted
 251 speed of 40 mph. The profile sampling interval was slightly different for each HSIP, which ranged
 252 between 0.6 and 0.9 in. Only the right HSIP laser and accelerometer data were used to measure
 253 faulting with the automated method. The data collection was interrupted on the first day due to rain
 254 but resumed on the following day. The weather was mostly fair on both days with partly cloudy
 255 skies.
 256

257 **ACCURACY AND PRECISION**

258 ***HSIP Performance***

259 To evaluate the performance of the HSIP it is important to compare the profiles directly, as
 260 index values may compare favorably even though profiles may not. The AASHTO PP-49 protocol
 261 sets a minimum profile repeatability and accuracy of 92% and 90%, respectively.
 262

263 In this experiment, each HSIP's profile repeatability was evaluated in terms of cross-correlation
 264 among three unfiltered replicate profiles (Table 1). All HSIP units met the minimum profile
 265 repeatability requirement except for HSIP 30781, whose operator was less experienced and had
 266 difficulty maintaining a consistent lateral position of the vehicle. Profile accuracy could not be
 267 evaluated since a profile reference device was not available during the study.
 268

269 The repeatability of profile measurement on diamond ground concrete depends heavily on the use
 270 of a large foot-print height sensor and consistent lateral tracking of the profiler [7]. However,
 271 since faulting measures the difference in elevation between points, the systematic error due to the
 272 bias of the relatively small laser sensor footprint is greatly reduced if not eliminated.

273

Phase 1

274 For this phase of the study, the faulting measurements by the HSIP at various speeds were
275 compared to the control measurements of the simulated faulting device. Figure 5 shows an
276 example of the insignificant effect of speed gradient on faulting under controlled conditions.
277

278

279 For this phase of the experiment, precision is expressed in terms of accuracy and
280 repeatability. The accuracy is the maximum faulting bias between the five HSIP and the simulated
281 faulting device. Repeatability is the maximum range in faulting within the five HSIP. Table 2
282 gives an example of assessing the simulated faulting repeatability and accuracy for one HSIP.
283 Table 3 provides a summary of the HSIP automated faulting precision under controlled conditions
284 at test speeds varying from 50 to 70 mph..
285

285

Phase 2

286 The main objective of the second phase was to perform a preliminary accuracy and
287 precision test of the automated faulting measurement method under normal field conditions. The
288 location of HSIP detected joints were compared to the 50 existing joints whose station locations
289 were determined with a measuring wheel. The average joint detection from three repeat passes of
290 each HSIP was expressed in terms of actual existing joints correctly detected (i.e. true positive),
291 existing joints not detected (i.e. true negative), non-existing joints detected (i.e. false positive), and
292 non-existing joints not detected (i.e. false negative). The joint detection rate of each HSIP was
293 measured by the ratio of positively detected joints as shown by the confusion matrix presented in
294 Table 4.
295

296

297 The average faulting estimated by each HSIP and the average manual faulting at the
298 detected joints are reported in Table 5. The bias is the average difference between the HSIP
299 estimated faulting and the average faulting at the detected joints. The precision for faulting
300 measurement is expressed in terms of accuracy, repeatability, and reproducibility (Table 6). The
301 accuracy is the average difference between the HSIP estimated faulting and the manually measured
302 faulting at all 50 joints; repeatability is expressed by the average standard deviation of all the
303 biases at all 50 joints; reproducibility is expressed as the maximum difference in faulting bias
304 among the five HSIP at any faulted joint.
305

305

ANALYSIS

306 Except for HSIP 30781, all other profilers passed the minimum profile repeatability cross-
307 correlation of 92% in both wheel-paths. Profile accuracy could not be verified since a reference
308 profiler was not available.
309

310 The results of Phase 1 show that speed gradient had a minimum effect on estimated faulting
311 under controlled conditions. Faulting measured by the HSIP under controlled conditions shows a
312 high degree of accuracy and repeatability as expressed by the relatively low faulting bias and range
313 of 0.60 mm and 0.65 mm, respectively. This is lower than the 1mm faulting resolution required by
314 the AASHTO R36 protocol.

315 The results from Phase 2 show that the proposed automated method yields a positive joint
316 detection rate ranging from 80 to 94%. Exception is made for HSIP 30781 with a positive joint
317 detection rate of 74%, which is mostly attributed to the inexperience of the operator. Under field
318 operating conditions, the average difference between faulting estimated by a HSIP and that

319 measured by a manual faultmeter was estimated at 1.2 mm. The average difference in estimated
320 faulting between any two independent runs of a single HSIP, was estimated at 1.1mm. The
321 maximum difference in estimated faulting between two different HSIP was estimated at 0.5 mm.
322 The variability in estimated faulting under field conditions is obviously larger than that estimated
323 under controlled conditions as was to be expected. The increased variability is due to a
324 combination of random factors including equipment, operator and pavement texture and vehicle
325 wander, most of which are greatly reduced under controlled conditions.
326

327 **CONCLUSIONS**

328 The present study was conducted primarily to assess the accuracy and precision of the
329 enhanced FDOT automated faulting method used in conjunction with a HSIP. A two-phase
330 approach was used for the validation process. The first phase focused on evaluating the accuracy
331 and repeatability of HSIP under controlled conditions. The second phase evaluated the automated
332 faulting method on a rigid pavement using five separate HSIP. The findings indicated the
333 following:
334

- 335 • Except for one HSIP, all profilers passed a minimum profile repeatability cross-correlation
336 of 92%
- 337 • Under controlled conditions, the HSIP has a faulting measurement accuracy and
338 repeatability of 0.60 mm and 0.65 mm, respectively.
- 339 • The HSIP has a positive joint detection rate ranging from 80 to 94%
- 340 • Under filed conditions, the HSIP has an accuracy, repeatability and reproducibility of 1.2
341 mm, 1.1mm, and 0.5 mm, respectively.
342

343 **RECOMMENDATIONS**

344 These initial findings suggest that the enhanced automated faulting measurement method
345 offers a safe and reliable alternative method for measuring faulting of jointed concrete pavements.
346 The simulated faulting test approach used in Phase 1 offers a practical method to test a HSIP's
347 ability to measure faulting under controlled conditions and to obtain an estimate of the systematic
348 error. Additional field testing will be required to take into account the variability introduced by
349 other factors such as different joint width, joint condition, slab curling which are typically
350 encountered when testing concrete pavements. This will result in a method with a much wider
351 application.
352

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357

358 **DISCLAIMER**

359 The content of this paper reflects the views of the authors who are solely responsible for
360 the facts and accuracy of the data as well as for the opinions, findings and conclusions presented
361 herein. The contents do not necessarily reflect the official views or policies of the Florida DOT.
362 This paper does not constitute a standard, specification, or regulation. In addition, the above listed
363 agency assumes no liability for its contents or use thereof.
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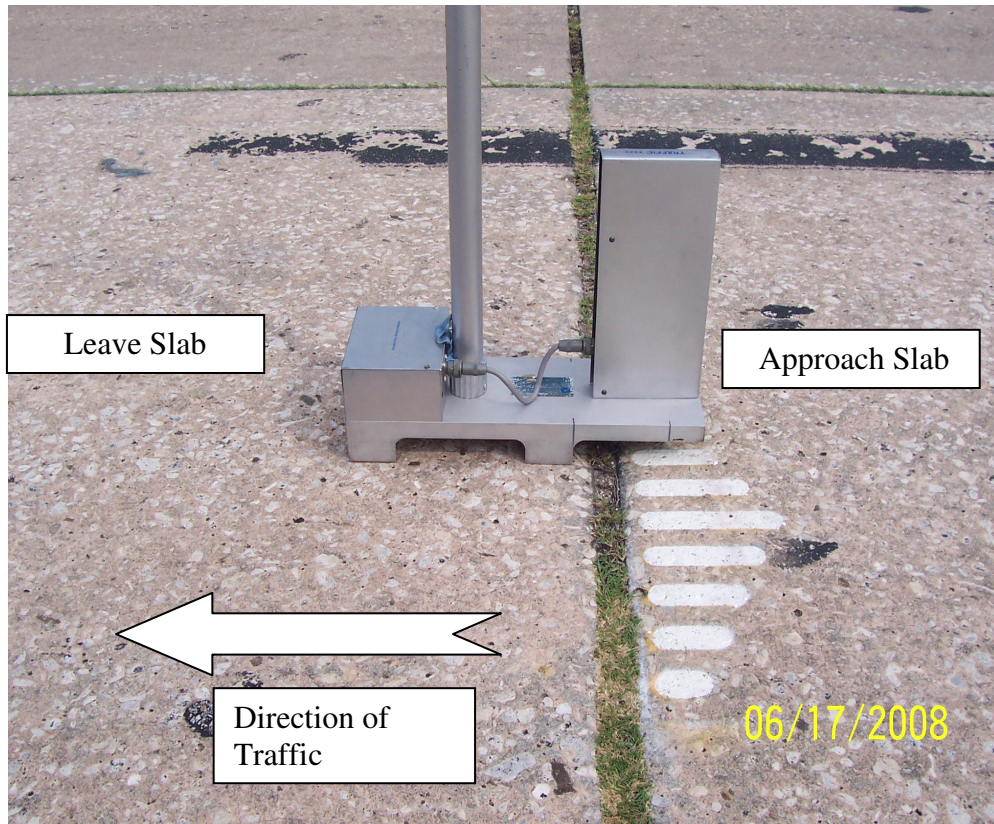
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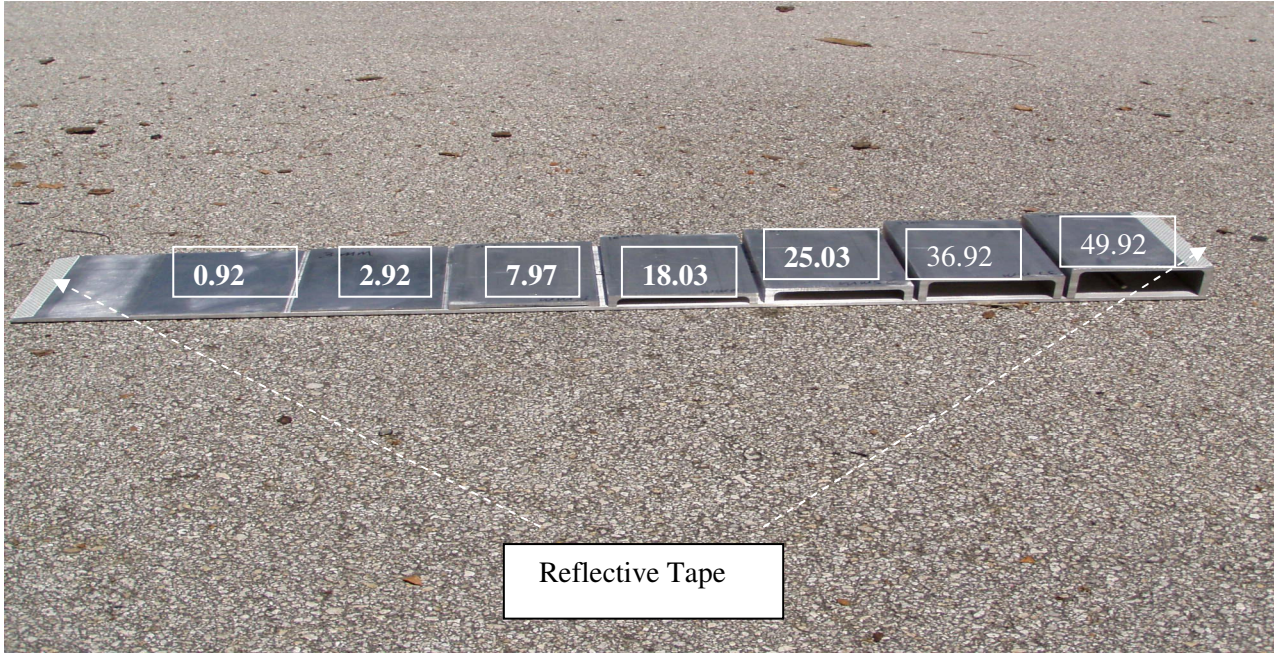
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FIGURE 1 Georgia Faultmeter



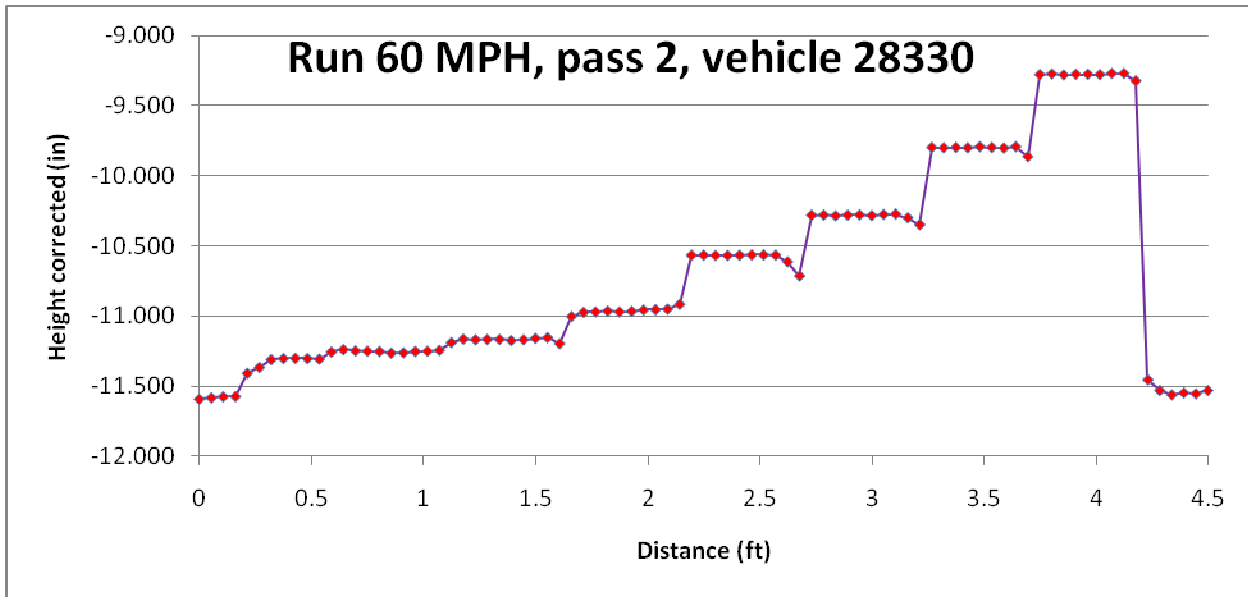
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FIGURE 2 FDOT High Speed Inertial Profiler (HSIP)



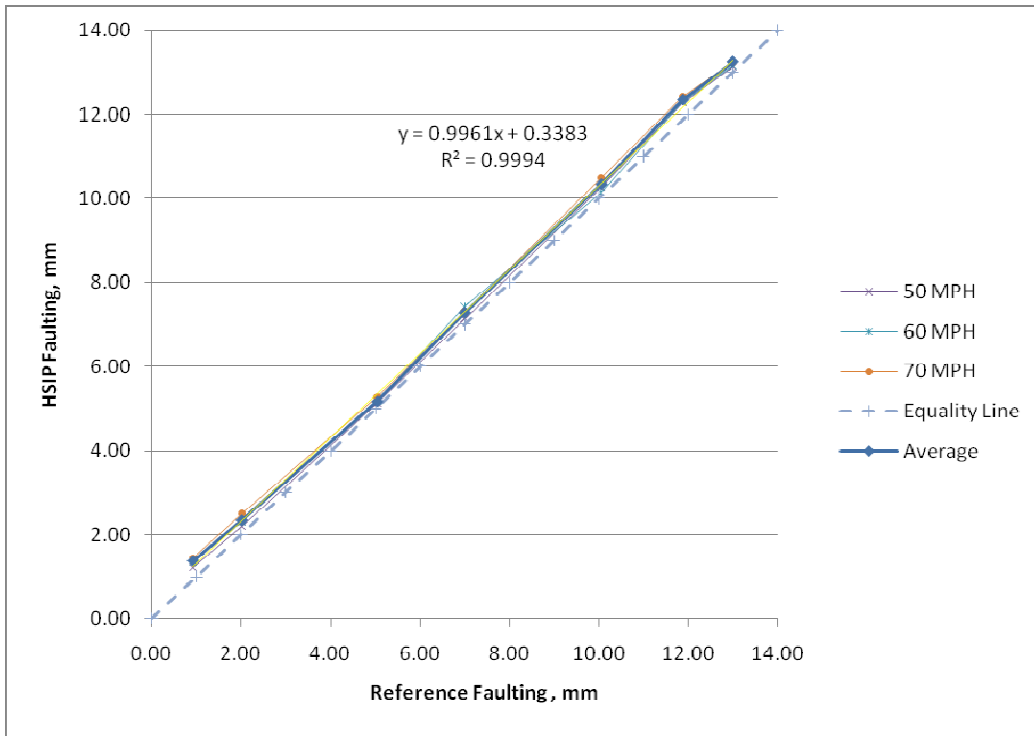
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FIGURE 3 Simulated Faulting Device



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FIGURE 4 Example of HSIP measured simulated faulting



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FIGURE 5 Effect of HSIP speed gradient on simulated faulting with eight degrees of freedom

TABLE 1 HSIP Profile Percent Cross-Correlation

HSIP No.	Sampling Interval (in)	Repeatability Left			Repeatability Right		
		Average	Minimum	Maximum	Average	Minimum	Maximum
29748	0.8	93	90	97	94	92	98
30330	0.7	92	90	94	96	95	98
30781	0.9	62	41	78	84	76	88
29863	0.8	94	92	96	97	96	97
30392	0.7	97	95	99	97	96	99

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TABLE 2 HSIP vs. Simulated Faulting, mm

HSIP 28330									
Reference Faulting	50 mph		60 mph		70 mph		Average Faulting	Maximum Range	Bias
	Mean	Range	Mean	Range	Mean	Range			
13.00	13.13	0.27	13.15	0.19	13.24	0.05	13.17	0.27	0.17
11.89	12.27	0.00	12.29	0.17	12.43	0.04	12.33	0.17	0.44
10.07	10.27	0.33	10.13	0.65	10.49	0.21	10.30	0.65	0.23
7.00	7.13	0.18	7.43	0.52	7.30	0.08	7.29	0.52	0.29
5.05	5.12	0.15	5.18	0.12	5.28	0.17	5.19	0.17	0.15
2.01	2.20	0.23	2.31	0.40	2.51	0.09	2.34	0.40	0.34
0.92	1.21	0.20	1.33	0.07	1.44	0.21	1.33	0.21	0.42

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TABLE 3 HSIP Simulated Faulting Precision

Precision, mm	
Accuracy	Repeatability
0.60	0.65

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TABLE 4 HSIP Joint Detection Matrix

Existing Joints	Average HSIP Detected Joints									
	29748		29863		30330		30781		30392	
	P	N	P	N	P	N	P	N	P	N
True	41	9	42	8	40	10	37	13	47	3
False	9	0	8	0	8	0	7	0	16	0
True Positive Rate (%)	82		84		80		74		94	

488
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490

P= Positive; N= Negative

491
492

TABLE 5 SR 331 Faulting Summary, mm

HSIP	Manual Faulting	Automated Faulting	Bias	St Dev
29748	2.1	2.0	0.9	0.18
29863	2.2	1.7	0.9	0.33
30330	2.2	1.9	1.0	0.34
30781	2.0	1.9	1.0	0.37
30392	2.2	1.6	1.0	0.25

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494

495

TABLE 6 HSIP Automated Faulting Precision

Precision (mm)		
Accuracy	Repeatability	Reproducibility
1.2	1.1	0.5

496