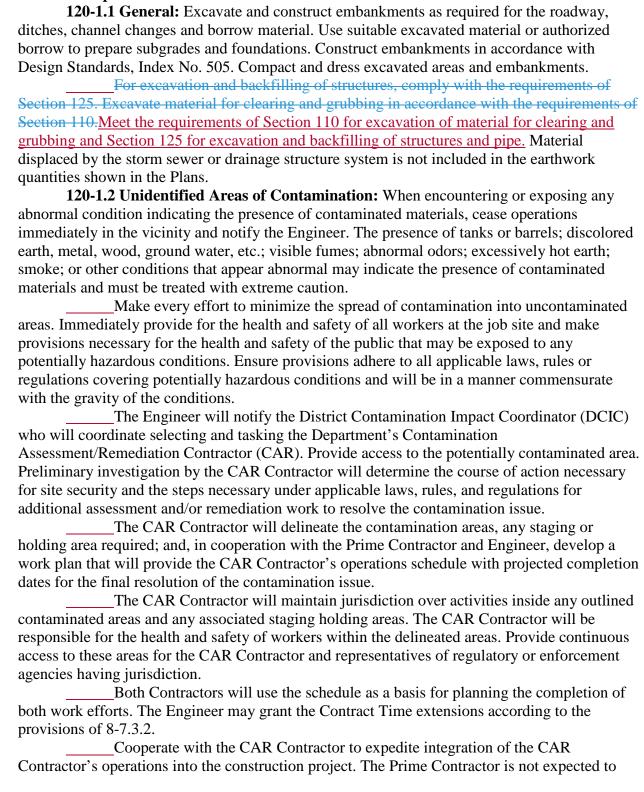
SECTION 120 EXCAVATION AND EMBANKMENT

120-1 Description.



engage in routine construction activities, such as excavating, grading, or any type of soil manipulation, or any construction processes required if handling of contaminated soil, surface water or ground water is involved. All routine construction activities requiring the handling of contaminated soil, surface water or groundwater will be by the CAR Contractor. Adjustments to quantities or to Contract unit prices will be made according to work additions or reductions on the part of the Prime Contractor in accordance with 4-3.

The Engineer will direct the Prime Contractor when operations may resume in the

120-2 Classifications of Excavation.

affected area.

120-2.1 General: The Department may classify excavation specified under this Section for payment as any of the following: regular excavation, subsoil excavation, lateral ditch excavation, and channel excavation.

_____If the proposal does not show subsoil excavation or lateral ditch excavation as separate items of payment, include such excavation under the item of regular excavation.

_____If the proposal shows lateral ditch excavation as a separate item of payment, but does not show channel excavation as a separate item of payment, include such excavation under the item of lateral ditch excavation. Otherwise, include channel excavation under the item of regular excavation.

120-2.2 Regular Excavation: Regular excavation includes roadway excavation and borrow excavation, as defined below for each.

120-2.2.1 Roadway Excavation: Roadway excavation consists of the excavation and the utilization or disposal of all materials necessary for the construction of the roadway, ditches, channel changes, etc., except as may be specifically shown to be paid for separately and that portion of the lateral ditches within the limits of the roadway right-of-way as shown in the Plans.

120-2.2.2 Borrow Excavation: Borrow excavation consists of the excavation and utilization of material from authorized borrow pits, including only material that is suitable for the construction of roadway embankments or of other embankments covered by the Contract.

_____A Cost Savings Initiative Proposal (CSIP) submittal based on using borrow material from within the project limits will not be considered.

120-2.3 Subsoil Excavation: Subsoil excavation consists of the excavation and disposal of muck, clay, rock, or any other material that is unsuitable in its original position and that is excavated below the finished grading template. For stabilized bases and sand bituminous road mixes, consider the finished grading template as the top of the finished base, shoulders and slopes. For all other bases and rigid pavement, consider the finished grading template as the finished shoulder and slope lines and bottom of completed base or rigid pavement. For pond and ditches that identify the placement of a blanket material, consider the finished grading template as the bottom of the blanket material. Subsoil excavation also consists of the excavation of all suitable material within the above limits as necessary to excavate the unsuitable material. Consider the limits of subsoil excavation indicated in the Plans as being particularly variable, in accordance with the field conditions actually encountered.

The quantity of material required to replace the excavated material and to raise the elevation of the roadway to the bottom of the template will be paid for under embankment or borrow excavation (Truck Measure).

120-2.4 Lateral Ditch Excavation: Lateral ditch excavation consists of all excavation of inlet and outlet ditches to structures and roadway, changes in channels of streams, and ditches

parallel to the roadway right-of-way. Dress lateral ditches to the grade and cross-section shown in the Plans.

120-2.5 Channel Excavation: Channel excavation consists of the excavation and satisfactory disposal of all materials from the limits of the channel as shown in the Plans.

120-3 Preliminary Soils Investigations.

When the Plans contain the results of a soil survey, do not assume such data is a guarantee of the depth, extent, or character of material present.

120-4 Removal of Unsuitable Materials and Existing Roads.

- **120-4.1 Subsoil Excavation:** Where muck, rock, clay, or other material within the limits of the roadway is unsuitable in its original position, excavate such material to the cross-sections shown in the Plans or indicated by the Engineer, and backfill with suitable material. Shape backfill material to the required cross-sections. Where the removal of plastic soils below the finished earthwork grade is required, meet a construction tolerance, from the lines shown in the Plans as the removal limits, of plus or minus 0.2 feet in depth and plus or minus 6 inches (each side) in width.
- **120-4.2 Construction over Existing Old Road:** Where a new roadway is to be constructed over an old one, plow or scarify the old road, and break it up full width, regardless of height of fill. If the Plans provide that paving materials may be incorporated into the fill, distribute such material in a manner so as not to create voids. Recompact the old road meeting the requirements of 120-10.2.
- **120-4.3 Obliterating Old Road:** Where the Plans call for obliteration of portions of an old road outside of the proposed new roadway, obliterate such sections of the old road by grading to fill ditches and to restore approximately the original contour of the ground or a contour which produces a pleasing appearance.

120-5 Disposal of Surplus and Unsuitable Material.

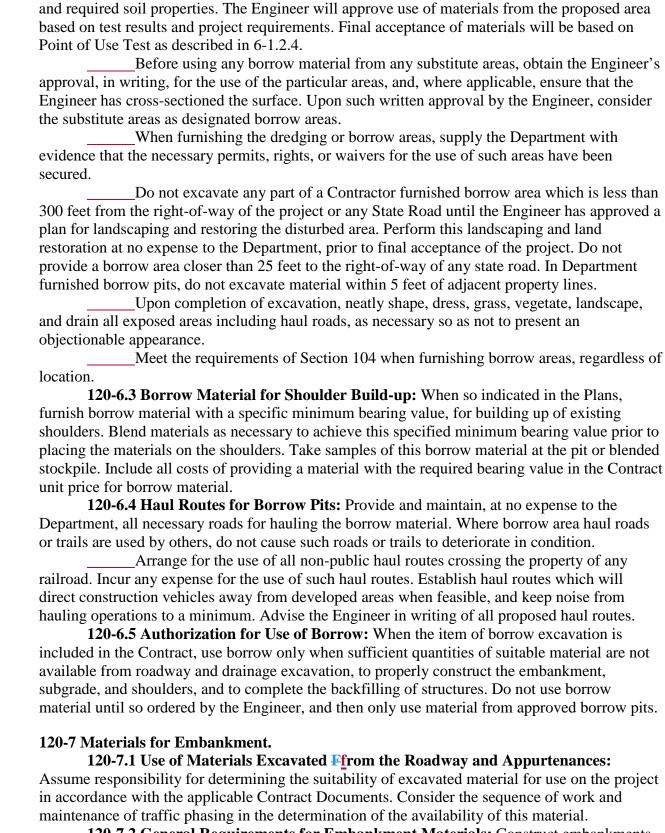
- **120-5.1 Ownership of Excavated Materials:** Dispose of surplus and excavated materials as shown in the Plans or, if the Plans do not indicate the method of disposal, take ownership of the materials and dispose of them outside the right-of-way.
- **120-5.2 Disposal of Muck on Side Slopes:** As an exception to the provisions of 120-5. 1, when approved by the Engineer, in rural undeveloped areas, the Contractor may place muck (A-8 material) on the slopes, or store it alongside the roadway, provided there is a clear distance of at least 6 feet between the roadway grading limits and the muck, and the Contractor dresses the muck to present a neat appearance. In addition, the Contractor may also dispose of this material by placing it on the slopes in developed areas where, in the opinion of the Engineer, this will result in an aesthetically pleasing appearance and will have no detrimental effect on the adjacent developments. Where the Engineer permits the disposal of muck or other unsuitable material inside the right-of-way limits, do not place such material in a manner which will impede the inflow or outfall of any channel or side ditches. The Engineer will determine the limits adjacent to channels within which such materials may be disposed.
- **120-5.3 Disposal of Paving Materials:** Unless otherwise noted, take ownership of paving materials, such as paving brick, asphalt block, concrete slab, sidewalk, curb and gutter, etc., excavated in the removal of existing pavements, and dispose of them outside the right-of-way. If the materials are to remain the property of the Department, place them in neat piles as directed. Existing limerock base that is removed may be incorporated in the stabilized portion of

the subgrade. If the construction sequence will allow, incorporate all existing limerock base into the project as allowed by the Contract Documents.

120-5.4 Disposal Areas: Where the Contract Documents require disposal of excavated materials outside the right-of-way, and the disposal area is not indicated in the Contract Documents, furnish the disposal area without additional compensation.

Provide areas for disposal of removed paving materials out of sight of the project and at least 300 feet from the nearest roadway right-of-way line of any State maintained road. If the materials are buried, disregard the 300 foot limitation.

120-6 Borrow.
120-6.1 Materials for Borrow: Do not open borrow pits until the Engineer has approved
their location.
Do not provide borrow materials that are polluted as defined in Chapter 376 of the Florida Statutes (oil of any kind and in any form, gasoline, pesticides, ammonia, chlorine, and
derivatives thereof, excluding liquefied petroleum gas) in concentrations above any local, State,
or Federal standards.
Prior to placing any borrow material that is the product of soil incineration,
provide the Engineer with a copy of the Certificate of Materials Recycling and Post Burn
Analysis showing that the material is below all allowable pollutant concentrations.
120-6.2 Furnishing of Borrow Areas: To obtain the Engineer's approval to use an off-
site construction activity area that involves excavation such as a borrow pit or local aggregate pit.
request in writing, a review for -cultural resources involvement. Send the request to the Division
of Historical Resources (DHR), Department of State, State Historic Preservation Officer,
Tallahassee, FL. As a minimum, include in the request the Project Identification Number, the
County, a description of the property with Township, Range, Section, etc., the dimensions of the
area to be affected, and a location map. Do not start any work at the off-site construction activity
area prior to receiving clearance from the DHR that no additional research is warranted.
For certain locations, the DHR will require a Cultural Resources Assessment
(CRA) Survey before approval can be granted. When this is required, secure professional
archaeological services to complete an historical and archaeological survey report. Submit the
report to the DHR and to the Department. The Engineer will determine final approval or
rejection of off-site construction activity areas based on input from the DHR.
Before receiving approval or before use of borrow areas, obtain written clearance
from the Engineer concerning compliance with the Federal Endangered Species Act and other
Wildlife Regulations as specified in 7-1.4 and Section 4(f) of the USDOT Act as specified in 7-
1.8.
The Department will adjust Contract Time in accordance with 8-7 for any
suspension of operations required to comply with this Article. The Department will not accept
any monetary claims due to delays or loss of off-site construction activity areas.
Except where the Plans specifically call for the use of a particular borrow or
dredging area, the Contractor may substitute borrow or dredging areas of his own choosing
provided the Engineer determines the materials from such areas meet the Department's standards
and other requirements for stability for use in the particular sections of the work in which it is to
be placed, and the Contractor absorbs any increase in hauling or other costs. Stake the corners of
the proposed borrow area and provide the necessary equipment along with an operator in order
for the Engineer to investigate the borrow area. The Engineer will determine test locations,
collect samples, and perform tests to investigate the proposed borrow area based on soil strata



120-7.2 General Requirements for Embankment Materials: Construct embankments of acceptable material including reclaimed asphalt pavement (RAP), recycled concrete aggregate

·	and cement concrete rubble, but containing no muck, stumps, roots, brush,
_	r, rubbish, reinforcement bar or other material that does not compact into a
	uring roadbed. Do not use RAP or RCA in the top 3 feet of slopes and shoulder
that are to be gra	assed or have other type of vegetation established. Do not use RAP or RCA in
stormwater mana	agement facility fill slopes.
R	demove all waste material designated as undesirable. Use material in
	nstruction in accordance with plan details or as the Engineer directs.
	Complete the embankment using maximum particle sizes (in any dimension) as
follows:	
	1. In top 12 inches: 3-1/2 inches (in any dimension).
	2. 12 to 24 inches: 6 inches (in any dimension).
	3. In the depth below 24 inches: not to exceed 12 inches (in any
dimension) or th	be compacted thickness of the layer being placed, whichever is less.
	pread all material so that the larger particles are separated from each other to
	between them during compaction. Compact around these rocks in accordance
with 120-9.2.	seemeen alem daring compaction. Compact around these rocks in accordance
	When and where approved by the Engineer, the Contractor may place larger rock
	8 inches in any dimension) outside the one to two slope and at least 4 feet or
	bottom of the base. Compact around these rocks to a firmness equal to that of the
	Construct grassed embankment areas in accordance with 120-9.2.65. Where
_	bankments adjacent to bridge end bents or abutments, do not place rock larger
	s in diameter within 3 feet of the location of any end-bent piling.
	Materials Used at Pipes, Culverts, etc.: Construct embankments over and
around pipes, cu	lverts, and bridge foundations with selected materials.
120 0 Emboules	ment Construction.
	General: Construct embankments in sections of not less than 300 feet in length
	ngth of the embankment. Do not construct another LOT over an untested LOT
	neer's approval in writing.
	or construction of mainline pavement lanes, turn lanes, ramps, parking lots,
	verts and retaining wall systems, a LOT is defined as a single lift of finished
	t to exceed 500 feet.
	or construction of shoulder-only areas, shared use paths, and sidewalks areas, a
LOT is defined a	as a single lift of finished embankment not to exceed 2000 feet.
Is	solated compaction operations will be considered as separate LOTSs. For
multiple phase co	onstruction, a LOT shall not extend beyond the limits of the phase.
120-8.2 I	Dry Fill Method:
12	20-8.2.1 General: Construct embankments to meet the compaction requiremen
	and in accordance with the acceptance program requirements in 120-10. Restric
the compacted th	nickness of the last embankment lift to 6 inches maximum.
r	As far as practicable, distribute traffic over the work during the
construction of e	embankments so as to cover the maximum area of the surface of each layer.
construction of c	Construct embankment using the dry fill method whenever normal
dewatering equit	Construct chibankincht using the dry fill inclide whenever helital
acwarding built	
as atoming equip	pment and methods can accomplish the needed dewatering.
	pment and methods can accomplish the needed dewatering. ———————————————————————————————————
	pment and methods can accomplish the needed dewatering.

Construct the embankment in successive layers with lifts up to a maximum <u>listed in the table</u> <u>below based on the embankment material classification group-compacted thickness of 12 inches.</u> <u>Ensure the percentage of fines passing the No. 200 US Standard sieve in the A 2 4 material does not exceed 15%.</u>

Group	AASHTO Soil Class	Maximum Lift Thickness	Thick Lift Control Test Section Requirements
<u>1</u>	<u>A-3</u> <u>A-2-4 (No. 200 Sieve ≤ 15%)</u>	12 inches	Not Needed
2	A-1 A-2-4 (No. 200 Sieve > 15%) A-2-5, A-2-6, A-2-7, A-4, A-5, A-6 A-7 (Liquid Limit < 50)	6 inches without Control Test Section	Maximum of 12 inches per 120-8.2.1.2

120-8.2.1.2 Thick Lift Requirements: For A-1, Plastic materials (as designated in Design Standard Index 505) and for A-2-4 Materials (with greater than 15% fines): Construct the embankment in successive layers with lifts up to a maximum compacted thickness of 6 inches.

Alternately, fFor embankment materials classified as Group 2 in the table above, the option to perform thick lift construction in successive layers of not more than 12 inches compacted thickness may be used after meeting the following requirements: -1. Notify the Engineer and obtain approval in writing prior to beginning construction of a test section. -a. Demonstrate the possession and control of compacting equipment sufficient to achieve density required by 120-10.2 for the full depth of a thicker lift. -2. Construct a test section of the length of one full LOT of not less than 500 feet. -3. Perform five Quality Control (QC) tests at random locations within the test section. -a. All five QC tests and a Department Verification test must meet the density required by 120-10.2. b. Identify the test section with the compaction effort and soil classification in the Department's Earthwork Records System (ERS). -4. Obtain Engineer's approval in writing for the compaction effort after completing a successful test section. A-1, Plastic material and for A-2-4 Materials (with greater than 15% fines), construct embankments using thick lift construction in successive layers of not more than 12 inches compacted thickness, after having demonstrated with a successful test section, the possession and control of compacting equipment sufficient to achieve density required by 120-10.2 for the full depth of a thicker lift, and if the Engineer approves the compaction effort. Notify the Engineer prior to beginning construction of a test section. Construct a test section of the length of one full LOT. Perform five QC tests at random locations within the test section. All

five QC tests and a Department Verification test must meet the density required by 120-10.2.

Identify the test section with the compaction effort and soil classification in the Density Log Book. In case of a change in compaction effort or soil classification, failing QC test or when the QC tests cannot be verified, construct a new test section. The Contractor may elect to place material in 6 inches compacted thickness at any time. Construct all layers approximately parallel to the centerline profile of the road.

The Engineer reserves the right to terminate the Contractor's use of thick lift construction. Whenever the Engineer determines that the Contractor is not achieving satisfactory results, revert to the 6 inch compacted lifts.

As far as practicable, distribute traffic over the work during the construction of embankments so as to cover the maximum area of the surface of each layer.

Construct embankment in the dry whenever normal dewatering equipment and methods can accomplish the needed dewatering.

120-8.2.1.3 Equipment and Methods: Provide normal dewatering equipment including, but not limited to, surface pumps, sump pumps and trenching/digging machinery. Provide normal dewatering methods including, but not limited to, constructing shallow surface drainage trenches/ditches, using sand blankets, sumps and siphons.

When normal dewatering does not adequately remove the water, the Engineer may require the embankment material to be placed in the water or on low swampy ground in accordance with 120-9.2.3.

120-8.2.2 Placing in Unstable Areas: Wheren depositing the fill material in water, or on low swampy ground that will not support the weight of hauling equipment, construct the embankment by dumping successive loads in a uniformly distributed layer of a thickness not greater than necessary to support the hauling equipment while placing subsequent layers. Once sufficient material has been placed so that the hauling equipment can be supported, construct the remaining portion of the embankment in layers in accordance with the applicable provisions of 120-9.2.2 and 120-9.2.4.

120-8.2.3 Placing on Steep Slopes: When constructing an embankment on a hillside sloping more than 20 degrees from the horizontal, before starting the fill, deeply plow or cut steps into the surface of the original ground on which the embankment is to be placed.

120-8.2.4 Placing Outside the Standard Minimum Slope: The standard minimum slope is defined as the plane described by a two (horizontal) to one (vertical) to two (horizontal) slope downward from the roadway shoulder pointline or the gutter line, as applicable in accordance with Design Standards, Index Nos. 500 and 505. Where material that is unsuitable for normal embankment construction is to be used in the embankment outside the standard minimum slope, place such material in layers of not more than 18 inches in thickness, measured loose. The Contractor may also place material which is suitable for normal embankment, outside such standard minimum slope, in 18 inch layers. Maintain a constant thickness for suitable material placed within and outside the standard minimum slope, unless placing in a separate operation.

120-8.3 Hydraulic Method:

120-8.3.1 Method of Placing: When the hydraulic method is used, as far as practicable, place all dredged material in its final position in the embankment by such method. Place and compact any dredged material that is rehandlworked, or moved and placed in its final position by any other method, as specified in 120-9.2. Baffles or any other form of construction may be used if the slopes of the embankments are not steeper than indicated in the Plans. The Contractor may use baffles or any form of construction he may select provided the slopes of the

embankments are not steeper than indicated in the Plans. Remove all timber used for temporary bulkheads or baffles from the embankment, and fill and thoroughly compact the holes thus formedall voids. When placing fill on submerged land, construct dikes prior to beginning of dredging, and maintain the dikes throughout the dredging operation.

120-8.3.2 Excess Material: Do not use <u>any</u> excess material placed outside the prescribed slopes<u>or</u>, below the normal high-water <u>level,table</u> to raise the fill <u>areas</u>. Remove only the portion of this material required for dressing the slopes.

120-8.3.3 Protection of Openings in Embankment: Leave openings in the embankments at the bridge sites. Remove any material which invades these openings or existing channels without additional compensation to provide the same <u>existing channel</u> depth <u>of channel</u> as <u>existed</u> before the construction of the embankment. Do not excavate or dredge any material within 200 feet of the toe of the proposed embankment.

120-8.4 Reclaimed Asphalt Pavement (RAP) Method:

120-8.4.1 General: Use only RAP material stored at facilities with an approved Florida Department of Environmental Protection Stormwater permit or, transferred directly from a milling project to the Department project. Certify the source if RAP material is from an identifiable Department project. Do not use RAP material in the following areas: construction areas that are below the seasonal high groundwater table elevation; MSE Wall backfill; underneath MSE Walls or the top 6 inches of embankment.

Prior to placement, submit documentation to the Engineer for his approval, outlining the proposed location of the RAP material.

120-8.4.2 Soil and RAP Mixture: Place the RAP material at the location and spread uniformly, using approved methods to obtain a maximum layer thickness of 4 inches. Mix this 4 inches maximum layer of RAP with a loose soil layer 8 to 10 inches thick. After mixing, meet all embankment utilization requirements of Design Standards, Index No. 505 for the location used. Do not mix RAP in the uppermost 12 inches in order to comply with 120-8.2.1. The total RAP and other embankment material shall not exceed 12 inches per lift after mixing and compaction if the contractor can demonstrate that the density of the mixture can be achieved. Perform mixing using rotary tillers or other equipment meeting the approval of the Engineer. The Engineer will determine the order in which to spread the two materials. Mix both materials to the full depth. Ensure that the finished layer will have the thickness and shape required by the typical section. Demonstrate the feasibility of this construction method by successfully completing a 500 foot long test section. For embankment construction, meet the requirements of 120-8. For compaction requirements of the soil and RAP mixture, meet the requirements of 120-9.

120-8.4.3 Alternate Soil and RAP Layer Construction: Construct soil in 6 to 12 inch compacted lifts and RAP in alternate layers with 6 inch maximum compacted lifts. Use soil with a minimum LBR value of 40 to prevent failure during compaction of the overlying RAP layer. Demonstrate the feasibility of this construction method by successfully completing a 500 foot long test section. For compaction requirements of both soil and RAP, meet the requirements of 120-9.

120-9 Compaction Requirements.

120-9.1 Moisture Content: Compact the materials at a moisture content such that the specified density can be attained. If necessary to attain the specified density, add water to the material, or lower the moisture content by manipulating the material or allowing it to dry, as is appropriate.

120-9.2 Compaction of Embankments:

- **120-9.2.1 General:** Uniformly compact each layer, using equipment that will achieve the required density, and as compaction operations progress, shape and manipulate each layer as necessary to ensure uniform density throughout the embankment.
- **120-9.2.2 Compaction Over Unstable Foundations:** Where the embankment material is deposited in water or on low swampy ground, and in a layer thicker than 12 inches (as provided in 120-8.2.2), compact the top 6 inches (compacted thickness) of such layer to the density as specified in 120-10.2.
- 120-9.2.3 Compaction Where Plastic Material Has Been Removed: Where unsuitable material is removed and the remaining surface is of the A-4, A-5, A-6, or A-7 Soil Groups (see AASHTO M145), as determined by the Engineer, compact the surface of the excavated area by rolling with a sheepsfoot roller exerting a compression of at least 250 psi on the tamper feet, for the full width of the roadbed (subgrade and shoulders). Perform rolling before beginning any backfill, and continue until the roller feet do not penetrate the surface more than 1 inch. Do not perform such rolling where the remaining surface is below the normal water table and covered with water. Vary the procedure and equipment required for this operation at the discretion of the Engineer.
- 120-9.2.4 Compaction of Material to Be Used in Base, Pavement, or Stabilized Areas: Do not compact embankment material which will be incorporated into a pavement, base course, or stabilized subgrade, to be constructed as a part of the same Contract.
- **120-9.2.45** Compaction of Grassed Shoulder Areas: For the upper 6 inch layer of all shoulders which are to be grassed, since no specific density is required, compact only to the extent directed.
- 120-9.2.56 Compaction of Grassed Embankment Areas: For Do not compact the outer layers of all ny embankments where plant growth will be established, do not compact. Leave this layer in a loose condition to a minimum depth of 6 inches for the subsequent seeding or planting operations. Do not place RAP or RAP blended material within the top 12 inches of areas to be grassed.
- **120-9.3 Compaction for Pipes, Culverts, etc.:** Compact the backfill of trenches to the densities specified for embankment or subgrade, as applicable, and in accordance with the requirements of 125-9.2.

Thoroughly compact embankments over and around pipes, culverts, and bridges in a manner which will not place undue stress on the structures, and in accordance with the requirements of 125-9.2.

120-9.4 Compaction of Subgrade: If the Plans do not provide for stabilizing, compact the subgrade (as defined in 1-3) in both cuts and fills, to the density specified in 120-10.2. <u>For cut areas, determine Standard Proctor Maximum Density in accordance with FM 1-T099 at a frequency of one per mile or when there is a change in soil type, whichever occurs first. For undisturbed soils, do not apply density requirements where constructing <u>narrow widening strips or paved</u> shoulders 5 feet or less in width.</u>

Where trenches for widening strips are not of sufficient width to permit the use of standard compaction equipment, perform compaction using vibratory rollers, trench rollers, or other type compaction equipment approved by the Engineer.

Maintain the required density until the base or pavement is placed on the subgrade.

120-10 Acceptance Program. 120-10.1 General Requirements: **120-10.1.1 Initial Equipment Comparison:** Before initial production, perform an initial nuclear moisture density gauge comparison test using with the QC, Verifications and Independent Assurance (IA) gauges. Unless the Engineer instructs, do not perform the initial equipment comparison more than once per project. When comparing the computed dry density of one nuclear gauge to a second gauge, three sets of calculations must be performed (IA to QC, IA to Verification, and QC to Verification). eEnsure that the difference between the any two computed dry densities does not exceed 2 lb/ft³ between gauges from the same manufacturer, and 3 lb/ft³ between gauges from different manufacturers. Repair or replace any QC gauge that does not compare favorably with the IA gauge.

Perform a comparison analysis between the QC nuclear gauge and the Verification nuclear gauge any time a nuclear gauge or repaired nuclear gauge is first brought to the project. Repair and replace any QC gauge that does not compare favorably with the Verification gauge at any time during the remainder of the project. Calibrate all QC gauges annually.

120-10.1.2 Initial Production LOT et: Before construction of any-other production LOT, prepare a 500 foot initial control section consisting of one full LOT. Notify the Engineer in writing at least 24 hours prior to production of the initial control section. Perform all QC tests required in 120-10.1.4. When the initial QC test results pass specifications, the Engineer will perform a Verification test to verify compliance with the specifications. Do not begin constructing another LOT until successfully completing the initial production LOT. The Engineer will notify the Contractor in writing of the initial production lotLOT approval within three working days after receiving the Contractor's QC data when test results meet the following conditions:

1. QC and Verification tests must meet the density

requirements specifications.

- 2. Verification test must meet the specifications.
- 3. Difference between QC and Verification computed dry density results shall meet the requirements of 120-10.1.1.

If Verification test result fails the density requirements of 120-10.2, correct the areas of non-compliance. The QC and Verification tests will then be repeated.

120-10.1.3 Density over 105%: When a QC computed dry density results in a value greater than 105% of the applicable Proctor maximum dry density, the Engineer will perform an Independent Verification (IV) density test within 5 feet. If the Independent Verification density results in a value greater than 105%, the Engineer will investigate the compaction methods, examine the applicable Standard Proctor Maximum Density and material description. The Engineer may collect and test an Independent Verification Standard Proctor Maximum Density sample for acceptance in accordance with the criteria of 120-10.2.

120-10.1.4 Quality Control (QC) Tests:

120-10.1.4.1 Standard Proctor Maximum Density Determination:

Determine the QC standard Proctor maximum density and optimum moisture content by sampling and testing the material in accordance with the specified test method listed in 120-10.2.

120-10.1.4.2 Density Testing Requirements: Ensure compliance to the requirements of 120-10.2 by Nuclear Density testing in accordance with FM 1-T238. Determine the in-place moisture content for each density test. Use FM 1-T238, FM 5-507 (Determination of Moisture Content by Means of a Calcium Carbide Gas Pressure Moisture Tester), or ASTM D-

4643 (Laboratory Determination of Moisture Content of Granular Soils by use of a Microwave Oven) for moisture determination. **120-10.1.4.3 Soil Classification:** Perform soil classification tests on the sample collected in 120-10.1.4.1, in accordance with AASHTO T88, T89, T90, and FM 1-T267. Classify soils in accordance with AASHTO M145 in order to determine compliance with embankment utilization requirements as specified in Design Standards, Index No. 505. Unless required by the Engineer, do not test or classify materials for stabilized subgrade or base. **120-10.1.5 Department Verification:** The Engineer will conduct Verification tests in order to accept all materials and work associated with 120-10.1.4. The Engineer will verify the QC results if they meet the Verification Comparison Criteria, otherwise the Engineer will implement Resolution procedures. The Engineer will select test locations, including Station, Offset, and Lift, using a Rrandom Nnumber generator, based on the LOTets under consideration. Each Verification test evaluates all work represented by the QC testing completed in those LOTs. In addition to the Verification testing, the Engineer may perform additional Independent Verification (IV) testing. The Engineer will evaluate and act upon the IV test results in the same manner as Verification test results. When the project requires less than four QC tests per material type, the Engineer reserves the right to accept the materials and work through visual inspection. 120-10.1.6 Reduced Testing Frequency: When no Resolution testing is required for 12 consecutive verified LOTs; or if required, the QC test data was upheld, reduce the QC density testing to one test every two LOTs by identifying the substantiating tests in the Density Log Book and notifying the Engineer in writing prior to starting reduced frequency of testing. Obtain the Engineer's written approval for the option to reduce density testing frequency to one test every two LOTs if Resolution testing was not required for 12 consecutive verified LOTs, or if Resolution testing was required, but the QC test data was upheld and all substantiating tests are recorded in the Earthwork Records System (ERS). Generate random numbers based on the two LOTs under consideration. When QC test frequency is reduced to one every two LOTs, obtain the Engineer's approval to place more than one LOT over an untested LOT. Assure similar compaction efforts for the untested LOTs. If the Verification test fails, and QC test data is not upheld by Resolution testing, the QC testing will revert to the original frequency of one QC test per LOT. Do not apply reduced testing frequency in construction of shoulder-only areas, shared use paths, and sidewalks, and first and last lift. 120-10.1.7 Payment for Resolution Tests: If the Resolution laboratory results compare favorably with the QC results, the Department will pay for Resolution testing. No additional compensation, either monetary or time, will be made for the impacts of any such testing. If the Resolution laboratory results do not compare favorably with the QC results, the costs of the Resolution testing will be deducted from monthly estimates. No additional time will be granted for the impacts of any such testing. 120-10.2 Acceptance Criteria: Obtain a minimum QC density of 100% of the standard Proctor maximum density as determined by FM 1-T099, Method C, with the following exceptions: embankment constructed by the hydraulic method as specified in 120-8.3; material

placed outside the standard minimum slope as specified in 120-8.2.4 except when a structure is

supported on existing embankment; and, other areas specifically excluded herein.

120-10.3 Additional Requirements:

120-10.3.1 Frequency: Conduct QC sampling and testing at a minimum frequency listed in the table below. The Engineer will perform Verification sampling and tests at a minimum frequency listed in the table below.

Test Name	Quality Control	Verification	Verification of Shoulder-Only Areas, Shared Use Paths, and Sidewalks
Standard Proctor Maximum Density	One per soil type	One per soil type	One per soil type
Density	One per LOT	One per four LOTS and for wet conditions, the first lift not affected by water	One per two LOTs
Soil Classification and Organic Content	One per Standard Proctor Maximum Density	One per Standard Proctor Maximum Density	One per Standard Proctor Maximum Density

Setations and offsets, using the random number generator approved by the Engineer. Do not use note-pads or work-sheets to record data for later transfer to the Density Log Book. Notify the Engineer upon successful completion of QC testing on each LOT <u>prior to placing another lift on top</u>.

120-10.4 Verification Comparison Criteria and Resolution Procedures:

120-10.4.1 Standard Proctor Maximum Density Determination: The Engineer will verify the QC results if the results compare within 4.5 lb/ft³ of the Verification test result. Otherwise, the Engineer will take one additional sample of material from the soil type in question. The State Materials Office (SMO) or an AASHTO accredited laboratory designated by the SMO will perform Resolution testing. The material will be sampled and tested in accordance with FM 1-T099, Method C.

The Engineer will compare the Resolution test results with the QC test results. If all Resolution test results are within 4.5 lb/ft³ of the corresponding QC test results, the Engineer will use the QC test results for material acceptance purposes for each LOT with that soil type. If the Resolution test result is not within 4.5 lb/ft³ of the Contractor's QC test, the Verification test result will be used for material acceptance purposes.

120-10.4.2 Density Testing: When a Verification or Independent Verification density test fails the Aacceptance Ceriteria, retest the site within a 5 feeoot radius and the following actions will be taken:

1. If the QC retest meets the <u>Aacceptance Ccriteria</u> and meets the 120-10.1.1 criteria when compared with the Verification or <u>Independent Verification</u> test, the Engineer will accept those LOTs.

2. If the QC retest does not meet the <u>Aa</u>cceptance <u>Ccriteria</u> and compares favorably with the Verification or <u>Independent Verification</u> test, rework and retest the LOT. The Engineer will re-verify those LOTs.

3. If the QC retest and the Verification or Independent Verification test do not compare favorably, complete a new comparison analysis as defined in 120-10.1.1. Once acceptable comparison is achieved, retest the LOTs. The Engineer will perform new verification testing. Acceptance testing will not begin on a new LOT until the Contractor has a gauge that meets the comparison requirements.

Record QC test results in the density log-book on approved Department forms provided by the Engineer. Submit the original, completed density log-book to the Engineer at final acceptance.

120-10.4.3 Soil Classification: The Engineer will verify the QC <u>test</u> results if the Verification and the QC test results to identify matching both match the soil utilization symbol <u>listed in the Design Standards Index No. 505 classifications</u>. Otherwise, the Engineer will test the <u>sample retained for Resolution testingtake one additional sample of material from the soil type in question</u>. The SMO or an AASHTO accredited laboratory designated by the SMO will perform the Resolution testing. The material will be sampled and tested in accordance with AASHTO T88, T89, and T90, and classified in accordance with AASHTO M145.

The Engineer will compare the Resolution test results with the QC test results. If the Resolution test matches the QC <u>soil utilization symbol classification</u>, the Engineer will use the QC <u>classification/soil utilization symbol</u> for material acceptance purposes. If the Resolution test result does not match the Contractor's QC <u>soil utilization symbol classification</u>, the Verification test results will be used for material acceptance purposes.

Verification test results satisfy the organic content test criteria in Design Standards, Index No. 505. Otherwise, the Engineer will test the sample retained for Resolution testing. The SMO or an AASHTO accredited laboratory designated by the SMO will perform Resolution testing. The material will be sampled and tested in accordance with FM 1-T267. If the Resolution test results satisfy the required criteria, material of that soil type will be verified and accepted. If the Resolution test results do not meet the required criteria, reject the material and reconstruct with acceptable material.

120-10.5 Disposition of Defective Materials: Assume responsibility for removing and replacing all defective material, as defined in Section 6.

_____Alternately, submit an Engineering Analysis Scope in accordance with 6-4 to determine the disposition of the material.

120-11 Maintenance and Protection of Work.

While construction is in progress, maintain adequate drainage for the roadbed at all times. Maintain a shoulder at least 3 feet wide adjacent to all pavement or base construction in order to provide support for the edges.

Maintain all earthwork construction throughout the life of the Contract, and take all reasonable precautions to prevent loss of material from the roadway due to the action of wind or water. Repair, at no expense to the Department except as otherwise provided herein, any slides, washouts, settlement, subsidence, or other mishap which may occur prior to final acceptance of the work. Perform maintenance and protection of earthwork construction in accordance with Section 104.

Maintain all channels excavated as a part of the Contract work against natural shoaling or other encroachments to the lines, grades, and cross-sections shown in the Plans, until final acceptance of the project.

120-12 Construction.

- **120-12.1 Construction Tolerances:** Shape the surface of the earthwork to conform to the lines, grades, and cross-sections shown in the Plans. In final shaping of the surface of earthwork, maintain a tolerance of 0.3 foot above or below the plan cross-section with the following exceptions:
- 1. Shape the surface of shoulders to within 0.1 foot of the plan-cross-section shown in the Plans.
- 2. Shape the earthwork to match adjacent pavement, curb, sidewalk, structures, etc.
 - 3. Shape the bottom of conveyance ditches so that the ditch impounds no water.
- 4. When the work does not include construction of base or pavement, shape the entire roadbed (shoulder point to shoulder point) to within 0.1 foot above or below the Plan cross-section.
- 5. When the work includes permitted linear stormwater management facilities, shape the swales and ditch blocks to within 0.1 feet of the cross-section shown in the Plans.

Ensure that the shoulder lines do not vary horizontally more than 0.3 foot from the true lines shown in the Plans.

120-12.2 Operations Adjacent to Pavement: Carefully dress areas adjacent to pavement areas to avoid damage to such pavement. Complete grassing of shoulder areas prior to placing the final wearing course. Do not manipulate any embankment material on a pavement surface.

_____When shoulder dressing is underway adjacent to a pavement lane being used to maintain traffic, exercise extreme care to avoid interference with the safe movement of traffic.

120-13 Method of Measurement.

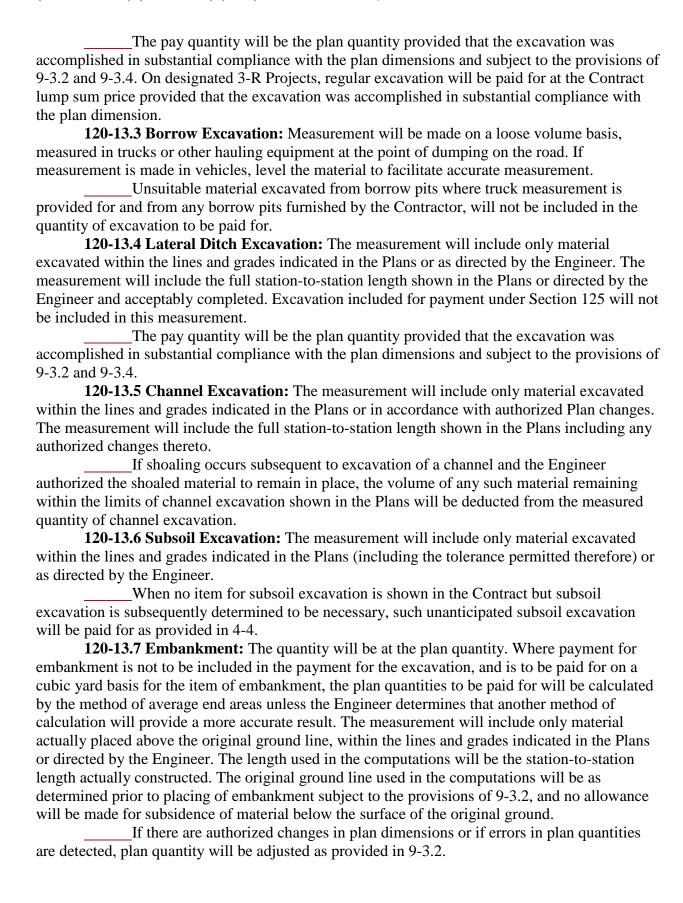
120-13.1 General: When payment for excavation is on a volumetric basis, the quantity to be paid for will be the volume, in cubic yards, calculated by the method of average end areas, unless the Engineer determines that another method of calculation will provide a more accurate result. The material will be measured in its original position by field survey or by photogrammetric means as designated by the Engineer, unless otherwise specified under the provisions for individual items.

Where subsoil excavation extends outside the lines shown in the Plans or authorized by the Engineer including allowable tolerances, and the space is backfilled with material obtained in additional authorized roadway or borrow excavation, the net fill, plus shrinkage allowance, will be deducted from the quantity of roadway excavation or borrow excavation to be paid for, as applicable.

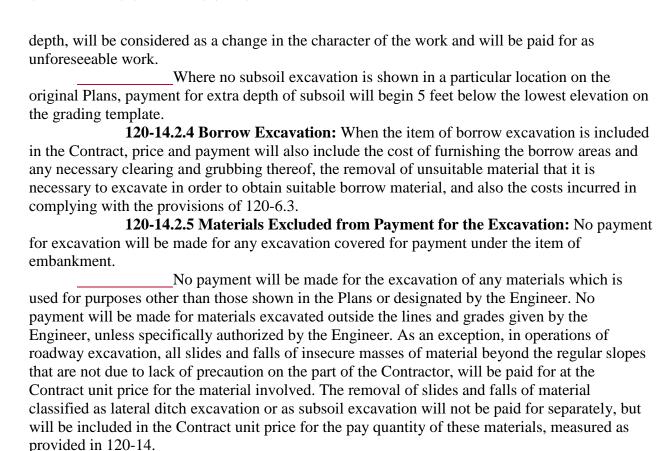
_____The quantity of all material washed, blown, or placed beyond the authorized roadway cross-section will be determined by the Engineer and will be deducted from the quantity of roadway excavation or borrow excavation to be paid for, as applicable.

Subsoil excavation that extends outside the lines shown in the Plans or authorized by the Engineer including allowable tolerances will be deducted from the quantity to be paid for as subsoil excavation.

120-13.2 Roadway Excavation: The measurement will include only the net volume of material excavated between the original ground surface and the surface of the completed earthwork, except that the measurement will also include all unavoidable slides which may occur in connection with excavation classified as roadway excavation.



Where the work includes excavation of unsuitable material below the finished				
grading template or original ground line, whichever is lower as defined in 120-3.3, the original				
ground line is defined as the surface prior to beginning excavation, except that this surface is not				
outside the permissible tolerance of lines and grades for subsoil excavation as indicated in the				
Plans or as directed by the Engineer. Any overrun or underrun of plan quantity for subsoil				
excavation which results in a corresponding increase or decrease in embankment will be				
considered as an authorized plan change for adjustment purposes as defined in 9-3.2.2.				
No payment will be made for embankment material used to replace unsuitable				
material excavated beyond the lines and grades shown in the Plans or ordered by the Engineer.				
In no case will payment be made for material allowed to run out of the				
embankment on a flatter slope than indicated on the cross-section. The Contractor shall make his				
own estimate on the volume of material actually required to obtain the pay section.				
120-14 Basis of Payment.				
120-14.1 General: Prices and payments for the various work items included in this				
Section will be full compensation for all work described herein, including excavating, dredging,				
hauling, placing, and compacting; dressing the surface of the earthwork; maintaining and				
protecting the complete earthwork; and hauling.				
The Department will not allow extra compensation for any rehandlworking of				
materials. The Department will compensate for the cost of grassing or other permanent erosion				
control measures directed by the Engineer as provided in the Contract for similar items of				
roadway work.				
120-14.2 Excavation:				
120-14.2.1 Items of Payment: When no classification of material is indicated in				
the Plans, and bids are taken only on regular excavation, the total quantity of all excavation				
specified under this Section will be paid for at the Contract unit price for regular excavation.				
When separate classifications of excavation are shown in the proposal, the				
quantities of each of the various classes of materials so shown will be paid for at the Contract				
unit prices per cubic yard for regular excavation, lateral ditch excavation, subsoil excavation, and				
channel excavation, as applicable, and any of such classifications not so shown will be included				
under the item of regular excavation (except that if there is a classification for lateral ditch				
excavation shown and there is no classification for channel excavation, any channel excavation				
will be included under the item of lateral ditch excavation). As an exception on designated				
projects, regular excavation will be paid for at the Contract lump sum price.				
120-14.2.2 Basic Work Included in Payments: Prices and payments will be full				
compensation for all work described under this Section, except for any excavation, or				
embankment which is specified to be included for payment under other items. Such prices and				
payments will include hauling; any rehandlwork ing that may be necessary to accomplish final				
disposal as shown in the Plans; the dressing of shoulders, ditches and slopes; removal of trash,				
vegetation, etc., from the previously graded roadway where no item for clearing and grubbing is				
shown in the Plans; and compacting as required.				
120-14.2.3 Additional Depth of Subsoil Excavation: Where subsoil excavation				
is made to a depth of 0 to 5 feet below the depth shown in the Plans, such excavation will be paid for at the unit price bid				
for at the unit price bid. Where subsoil excavation is made to a depth greater than 5 feet, and up to				
Where subsoil excavation is made to a depth greater than 5 feet, and up to 15 feet, deeper than the depth shown in the Plans, such excavation will be paid for at the unit				
price bid plus 25% of such unit price. Additional extra depth, more than 15 feet below such plan				
price ora pras 25% or such unit price. Additional extra depth, more than 15 feet below such plan				



120-14.3 Embankment:

120-14.3.1 General: Price and payment will be full compensation for all work specified in this Section, including all material for constructing the embankment, all excavating, dredging, pumping, placing and compacting of material for constructing the embankment complete, dressing of the surface of the roadway, maintenance and protection of the completed earthwork, and the removal of rubbish, vegetation, etc., from the roadway where no clearing and grubbing of the area is specified in the Plans. Also, such price and payment, in each case, will specifically include all costs of any roadway, lateral ditch, or channel excavation, unless such excavation is specifically shown to be paid for separately, regardless of whether the materials are utilized in the embankment.

120-14.3.2 Excluded Material: No payment will be made for the removal of muck or overburden from the dredging or borrow areas. No payment will be made for embankment material used to replace muck or other unsuitable material excavated beyond the lines and grades shown in the Plans or ordered by the Engineer.

120-14.3.3 Clearing and Grubbing: No payment will be made for any clearing and grubbing of the borrow or dredging areas. Where no clearing and grubbing of such areas is specified in the Plans, the cost of any necessary clearing and grubbing will be included in the Contract unit or lump sum price for Embankment.

120-14.3.4 Cost of Permits, Rights, and Waivers: Where the Contractor provides borrow or dredging areas of his own choosing, the cost of securing the necessary permits, rights or waivers will be included in the Contract price for embankment.

120-14.4 Payment Items: Payment will be made under:

Item No. 120- 1- Regular Excavation - per cubic yard.

(REV 7-19-17) (FA 8-1-17) (1-18) includes 1201201, 1200000

Item No. 120- 2-	Borrow Excavation - per cubic yard.
Item No. 120- 3-	Lateral Ditch Excavation - per cubic yard.
Item No. 120- 4-	Subsoil Excavation - per cubic yard.
Item No. 120- 5-	Channel Excavation - per cubic yard.
Item No. 120- 6-	Embankment - per cubic yard.
Item No. 120-71-	Regular Excavation (3-R Projects) - lump sum.