

# **SR 29 ENGINEERING REPORT**

SR 29 FROM SR 78 TO US 27

GLADES COUNTY ROADWAY ID: 05090000 MILEPOINT: 1.850 – 12.393



April 6, 2022

**District 1** 



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# 1.0 INTRODUCTION

On September 22, 2021, FDOT Heartland Operations Center was notified of water overtopping SR 29 in Glades County. Upon further field investigation the roadway had to be closed to traffic due to safety concerns with the standing water on the roadway. FDOT District One Drainage conducted a field review on this date to determine the extent of the flooding and the potential cause of the issue.

The flooding limits were observed to take place between mile points 4.850 and 6.380. For the purposes of this report, SR 29-mile points 1.85 (south end of SR 29, defined by a bridge #050941) to 12.393 (north end of SR 29, defined by railroad tracks) were taken into account.

#### Google Maps: Google Maps



Figure 1: Location Map



# 2.0 EXISTING CONDITIONS

SR 29 is a 2-lane undivided rural roadway with 12-foot travel lanes and 5-foot outside paved shoulders. This corridor is listed as Rural Principle Arterial Other with a roadway ID of 05090000. Currently the posted speed is 60 mph. This section of SR 29 is listed as a regional freight corridor, part of the state of Florida's Strategic Intermodal System (SIS), and a hurricane evacuation route. A straight-line diagram for the corridor is included in **Appendix A** and an evacuation route map is included in **Appendix B**.

The Annual Average Daily Traffic (AADT) along SR 29 is presented in **Table 1** below along with the portable traffic monitoring stations (PTMS) within the vicinity of the study segment. The most recent traffic count year for monitoring data is from 2020. The percentage of daily truck traffic (T-Factor) is approximately 30 percent.

Street	PTMS	Count Year	AADT	T-Factor
SR 29 (CR 74 to US 27)	051001	2020	4,600	29.00
SR 29 (SR 78 to CR 74)	050008	2020	3,600	30.10
SR 29 (CR 720 to SR 78)	051000	2020	6,100	30.10

#### Table 1: Segment AADT

### 2.1 Intersections

There are two main Stop Sign controlled intersections within the study area. These are at SR 78 (MP 2.505) and CR 74 (MP 11.676). Intersection photos are located in **Appendix C** for all approaches.

The intersection of SR 78 and SR 29 is a T-intersection with a Stop Sign controlling traffic coming from SR 78 entering onto SR 29. The main road at this intersection is SR 29 which has continuous through lanes supported by a separate northbound right turn lane and southbound left turn lane. SR 78 connects to SR 29 from the east.

The intersection of CR 74 and SR 29 is a T-intersection with a Stop Sign controlling traffic coming from CR 74 entering onto SR 29. The main road at this intersection is SR 29 which has continuous through lanes supported by a separate northbound left turn lane. CR 74 connects to SR 29 from the west.

## 2.2 Bridges

There are four bridges located within the corridor. See **Table 2** for details. All these bridges have exceeded their 50-year design service life as they were originally constructed in 1948 (74 years old) and were widened late into their service life. The NBI Ratings of the existing bridges is showing signs of deterioration and the need for increased maintenance to maintain their integrity. Additionally, the existing bridges are essentially at grade with the existing roadway making them susceptible to inundation and not meeting today's standard 2-foot drift clearance for debris. Additionally, the low vertical clearance is not conducive to wildlife movement, forcing wildlife onto the roadway in order to cross SR 29.



Bridge #	# Name Year Mile Points Built		Mile Points	Year Reconstruction	Sufficiency Rating	NBI Ratings
050035	SR 29 over Lone Pine Creek	1948	4.709 to 4.723	1999-Bridge was widened (FPID 193991-1)	80.7	Deck: 7 Superstructure: 7 Substructure: 7
050033	SR 29 over Cypress Branch (aka Chapparal Slough)	SR 29 over Cypress Branch (aka 1948 6.851 to 6.880 Chapparal Slough)		1978 – Bridge was widened	79.3	Deck: 6 Superstructure: 6 Substructure: 6
050032	SR 29 over York Branch	1948	9.353 to 9.362	1999- Bridge was widened (FPID 193991-1)	70.98	Deck: 7 Superstructure: 7 Substructure: 6
050031	SR 29 over Turkey Branch	1948	10.931 to 10.937	1999- Bridge was widened (FPID 193991-1)	79	Deck: 7 Superstructure: 7 Substructure: 7





Figure 2: Existing Bridge Locations



## 2.3 Pavement Condition

This section of SR 29 last had a pavement condition survey on 3/3/2021. The results of this survey are shown below in **Table 3**. A rating of 6 or below is considered deficient. The crack rating on this segment is deficient and is listed as having transverse cracking. This indicates that this road needs resurfacing in the near future. The survey also showed that there is rippling, depressions, and delamination of the existing pavement. The road surface is currently paved with FC-12.5 Dense graded friction course.

Section	BMP	EMP	Lane	Crack	Rut	Ride
05090000	0.000	2.621	R1	6	8	7.9
05090000	2.621	12.437	R1	5.5	8	7.8

Table 3: Pavement Condition Survey Results 2021

A roadway retro reflectivity survey for this segment was last conducted on 6/4/2021. Values of 250 mcd/m2 /lux or above are rated as good and easily visible, while a rating between 150 and 250 mcd/m2 /lux is seen as adequate but on the verge of needing replacement, and values of 150 mcd/m2 /lux and below are in poor condition and in need of restriping. The average results of the study segment from the most recent survey are shown below in **Table 4**. As seen in the data, the existing striping needs replacement to increase the retro reflectivity in the corridor. See **Appendix D** for a more detailed breakdown of reflectivity ratings throughout the corridor.

#### Table 4: Pavement Markings Retro Reflectivity Survey 2021

EMP

RCL

BMP

Date



**Figure 3: Reflectivity Labels** 



Roadway skid tests are completed once every 4 years on state roadways. The skid number (friction number) is a measure of how good the pavement will perform when it is wet. A skid number less than 31 is seen as deficient in the amount of friction a tire can have with the roadway. The most recent skid test for this segment was conducted on 1/6/2020 as shown in **Table 5** below. Based on the results of this test the roadway friction is still in the satisfactory range. For a more detailed breakdown of the skid ratings report see **Appendix E**.

Date	Direction	Milepost	Average Friction Number	Mean Profile Depth Average (in.)
1/6/2020	Left (south)	0.000 - 2.637	46	0.046
1/6/2020	Left (south)	2.637 – 12.437	44	0.029

#### Table 5: Roadway Skid Test Results 2020

### 2.4 Past Projects

- 1. Bridge #050033 was widened in 1978 as art of project 05090-3504. The existing bridge structure remained in place and was not reconstructed as part of this project.
- 2. Bridges #050031, #050032, and #050035 were widened as part of FPID 193991-1 in 1999. The existing bridge structures remained in place and were not reconstructed as part of this project.
- 3. The majority of the study segment was last resurfaced in 2004 as part of FPID 193957-2-52-01. The limits of this project were from MP 2.621 to MP 12.437.
- 4. The most recent drainage work for the study segment was completed in 2013 as part of FPID 431394-1-52-01. See section 4.3 of this report for more details.

## 3.0 CRASH ANALYSIS

Crash data from March 29<sup>th</sup>, 2015 to March 29<sup>th</sup>, 2022 was extracted from the Signal 4 Analytics and SSOGis database. A total of 108 crashes were reported along the study corridor during the study period but after review only 99 were seen to have taken place in the study area. 16 of these occurred during wet weather. There were 5 crashes involving a fatality (see **Table 6**) and 22 crashes which resulted in injuries. An overview of the crash history for this period is presented in **Table 7**. See **Appendix F** for fatal crash reports.

## 3.1 Crash Details

<u>Crash number 85192700</u> resulted in a fatality. This crash occurred on 11/7/2015 around 4:35 pm in the northbound travel lane of SR 29 just south of SR 78. The crash was listed as "unknown" for the type of crash and took place during wet weather. Vehicle 1 was southbound on SR 29 when the driver lost control of the vehicle and rotated across the centerline into the northbound lane. Vehicle 1 struck vehicle 2 in the northbound lane. The driver of vehicle 2 was pronounced deceased. Vehicle one was found to have deficient tire tread.

<u>Crash number 85605345</u> resulted in a fatality. This crash occurred on 11/22/2017 around 5:35 am in the northbound travel lane of SR 29. The crash was listed as "head on" for the type of crash. Vehicle 1 was southbound on SR 29 north of Chaparral Avenue and vehicle 2 was northbound on SR-29. Vehicle 1 crossed into the northbound lane striking the front of vehicle 2. The driver of vehicle 1, driver of vehicle 2, and a passenger of vehicle 2 were pronounced deceased.

<u>Crash number 85455740</u> resulted in a fatality. This crash occurred on 12/28/2017 around 1:53 pm in the southbound travel lane of SR 29. The crash was listed as "other" for the type of crash. Vehicle 1 was traveling northbound on SR 29 north of Chapparal Avenue and made an improper U-turn in front of vehicle 2 in the southbound lane. Vehicle 2 struck vehicle 1. Driver of vehicle 1 was pronounced deceased.

<u>Crash number 87177808</u> resulted in a fatality. This crash occurred on 7/3/2018 around 3:11 pm in the northbound travel lane of SR 29. The crash was listed as "head on" for the type of crash and took place during wet weather. Vehicle 1 was southbound on SR 29 and vehicle 2 was northbound on SR 29 near Chapparal Avenue. Vehicle 1 crossed over the centerline and struck the front of vehicle 2. The driver of vehicle 1, two passengers in vehicle 2 were pronounced deceased.

<u>Crash number 88371042</u> resulted in a fatality. This crash occurred on 7/15/2021 around 2:13 am in the northbound travel lane of SR 29. The crash was listed as "head on" for the type of crash. Vehicle 1 was travelling southbound on SR 29 south of Chapparal Avenue and vehicle 2 was traveling northbound on SR 29. Vehicle 1 crossed the centerline and collided with the front of vehicle 2. Driver of vehicle 1 was pronounced deceased.

<u>Crash number 85463418</u>was listed as "other" for the type of crash. This crash occurred during wet weather. Vehicle 2 was a tractor trailer which tried to make a wide right turn onto SR 78 while traveling northbound on SR 29. Vehicle 1 failed to stop and ran into the side of vehicle 2.

<u>Crash number 89452684</u> was listed as "other" for the type of crash. Vehicle 1 was towing a trailer heading southbound on SR 29. One of the tires on the left side of the trailer fell off and went into the northbound lane. Vehicle 2 collided with the tire causing the vehicle to be disabled.

<u>Crash number 87104488</u> was listed as "other" for the type of crash. Vehicle 1 was traveling southbound on SR 29 and towing a trailer when the wheels fell off and traveled into the northbound lane. One of the wheels collided with vehicle 2 in the northbound lane and the other wheel collided with vehicle 3 in the northbound lane.

<u>Crash number 82771435</u> was listed as "other" for the type of crash. Vehicle was traveling south on US 27 when it turned onto SR 29 and struck the railroad crossing arm causing significant damage. Vehicle 1 fled the scene prior to arrival of law enforcement.

<u>Crash number 82771243</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 87168190</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 87920439</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.



<u>Crash number 89452679</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 89452680</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 85561725</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 24400882</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 88428603</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

<u>Crash number 82771016</u> was coded incorrectly and is not located within the study segment. This crash was not used for statistical purposes.

Crash Number	Crash Date	Crash Time	Direction	Road Surface	Crash Type	Number of Fatalities
85192700	11/7/2015	4:35 PM	South	Wet	Unknown	1
85605345	11/22/2017	5:35 AM	North	Dry	Head On	3
85455740	12/28/2017	1:53 PM	North	Dry	Other	1
87177808	7/3/2018	3:11 PM	South	Wet	Head On	2
88371042	7/15/2021	2:13 AM	South	Dry	Head On	1

#### Table 6: Fatal Crash Summary

#### 3.2 Crash Data





\*2015 and 2022 data is only a partial year



Cra	Crash Year								0/	Total	
Сга	SITILIOITIALIOIT	2015	2016	2017	2018	2019	2020	2021	2022	/0	TOLAT
	Angle	0	1	0	0	0	0	1	1	3.0%	3
	Rear End	0	2	0	1	0	1	0	0	4.0%	4
	Left Turn	0	1	0	1	2	1	1	0	6.1%	6
	Backed-into	0	0	0	0	0	0	0	0	0.0%	0
	Right Turn	0	0	1	0	0	0	0	0	1.0%	1
	Sideswipe	1	1	1	0	0	0	1	0	4.0%	4
Crach Tuna	Head On	0	0	1	1	0	0	2	1	5.1%	5
Crash Type	Run-off-Road	1	0	4	5	1	3	5	0	19.2%	19
	Rollover	0	2	0	2	0	1	1	0	6.1%	6
	Animal	2	2	3	4	4	7	5	0	27.3%	27
	Pedestrian	0	0	0	0	0	0	0	0	0.0%	0
	Bicycle	0	0	0	0	0	0	0	0	0.0%	0
	Other/unknown	1	5	4	5	4	5	0	0	24.2%	24
	Total	5	14	14	19	11	18	16	2	100%	99
	Fatal	1	0	2	1	0	0	1	0	5.1%	5
Injury	Injury	2	6	2	6	1	2	1	2	22.2%	22
Seventy	Property Damage Only	2	8	10	12	10	16	14	0	72.7%	72
Lighting	Daylight	1	9	4	7	4	8	3	1	37.4%	37
Condition	Night	4	5	10	12	7	10	13	1	62.6%	62
Surface	Dry	3	13	10	16	11	15	13	2	83.8%	83
Conditions	Wet	2	1	4	3	0	3	3	0	16.2%	16

Table 7: Crash Summary



Figure 5: Crashes By Time of Day



## Figure 6: Distribution of All Crash Types from March 2015 to March 2022

## 3.3 Crash Analysis

Analysis of the crashes along the corridor shows that the majority of crashes happen at night and are animal collisions. There is currently no street lighting along the corridor due to this being a rural road. SR 29 does not currently contain any wildlife fencing to help prevent animals from entering the roadway and the existing bridges along the corridor don't have sufficient vertical clearance conducive for wildlife passage. Upon reading the crash reports it appears that wild hogs are the most commonly hit animal. There were 5 fatality crashes in the corridor, and all of these were from cross centerline head on crashes. This section of road does not currently have centerline rumble strips or shoulder rumble strips.



# 4.0 FLOODING ASSESSMENT

## 4.1 Existing Drainage Conditions

The drainage in this corridor is an open system of grass ditches and swales which cross SR 29 via 2 cross drains and at the existing bridges. There is no formal water quality treatment or attenuation for the SR 29 stormwater runoff. The roadway is a normal crown with the southbound lanes flowing toward the west ditch and the northbound lanes flowing toward the east ditch. The general sheet flow pattern in this area is from northwest to southeast toward SR 78 and the Caloosahatchee River. There are 2 main outfalls for SR 29 and 1 smaller culvert all located to the south along SR 78. These are located at Deadmans Branch and Cypress Branch see **Figure 7**. The ultimate outfall is at the Caloosahatchee River. This project is located in the 100-year floodplain. See **Appendix G** for floodplain map.



Figure 7: Existing Drainage Patterns



## 4.2 Historical Flooding Events







#### Figure 9: Flooding August 26, 1999







Figure 10: Hurricane Irma (2017) SR 29 Flooding Photo 1

Figure 11: Hurricane Irma (2017) SR 29 Flooding Photo 2



Figure 12: SR 29 Flooding September 22, 2021 Photo 1





Figure 13: SR 29 Flooding September 22, 2021 Photo 2

## 4.3 Past Drainage Reports and Projects

#### • <u>1999 Drainage Complaint</u>

*Findings:* Water was not overtopping the roadway, but the bridges were flowing full right up to the low member of the structure.

*Recommendation:* To correct the potential issue of roadway flooding the existing roadway profile will need to be raised. See **Appendix H** for the full report.

#### January 2010 Drainage Complaint Report completed by AIM Engineering & Surveying, Inc.

*Findings:* The Triple 30" Cross Drain (MP) appeared to be undersized and both cross drains had severe silting blocking the flow

*Recommendation:* Permanent solution is to raise the roadway profile. Short term solution was to desilt the existing pipes and dig out the existing roadside ditches. Recommended to also remove and replace the existing triple 30" cross drain with quadruple 48" pipes. See **Appendix H** for the full report.



#### • Pond Siting Report for SR 29 From CR 80A to US 27 FPID 417878-1-22-01 April 2011

*Purpose:* The PD&E for this section of SR 29 was originally scheduled to look at potential 4 lane widening through the corridor. The study area for this PD&E was reduced to not being able to show a purpose and need for the 4-lane widening on the northern rural section of SR 29. Therefore, no improvements to the study area for this engineering report were made based on the PD&E findings.

#### • Drainage Improvement Project FPID 431394-1-52-01

*Purpose:* Re-grade and excavate the existing roadside ditches on SR 29 from MP 2.736 (SR 78) to 8.846. The two cross drains had pipe liners installed. See **Appendix I** for project typical section

#### 4.4 Rainfall

This location falls within zone 8 of the state of Florida IDF curves. The average monthly rainfall from NOAA is shown in **Table 8**. The wet season takes place between the beginning of May to the end of September. The yearly rainfall from NOAA is shown in **Table 9**. See **Appendix J** for Historical rainfall data.

#### Table 8: Normal Average Monthly Rainfall (NOAA)

Month	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
Rainfall (inches)	1.5 - 2	2 - 3	3 - 4	2 - 3	3 - 4	8 - 10	6 - 8	8 - 10	6 - 8	3 - 4	2 - 3	1.5 – 2

#### Table 9: Yearly Total Rainfall (NOAA)

Year	2015	2016	2017	2018	2019	2020	2021	Normal
Rainfall (inches)	60 - 70	70 - 80	60 - 70	50-60	50 - 60	50 - 60	50-60	50 - 60

#### 4.5 Flooding Analysis

Based on historical accounts and flood records provided by the Operations staff, SR 29 floods every few years, requiring the road to be closed. FDOT completed a project (FPID 431394-1-52-01) in 2013 to help minimize the flooding by widening the ditches on both sides of SR 29 to provide more capacity. While this helped to reduce the frequency of SR 29 flooding and road closures, there have been several closures since the project was completed. From witness accounts, it was noted that the triple 30" cross drain located at MP 5.528 tends to be the first drainage structure to overflow. On September 22, 2021 during a field review the limits of the roadway overtopping were observed to take place between MP 4.850 and MP 6.380. Water was seen to be backing up at both cross drains (MP 5.528 and MP 6.267) and neither structure was visible. As can be seen in **Figure 13**, there was a large amount of sheet flow coming from the west side of SR 29 and much of the adjacent land was inundated by water.





Figure 14: West side of Cross Drain at MP 5.528





#### Figure 15: Flooding Limits September 22, 2021

In attempts to address the recurring flooding issues, in 2013 FDOT completed the drainage improvement project (FPID 431394-1-52-01) described above, providing more capacity within the roadside ditches. While this helped to reduce the frequency of SR 29 flooding and road closures, there have been several closures since the project was completed.

For this report, an analysis of the capacity of the existing triple 30" cross drain was performed using the FHWA HY-8 Culvert Hydraulic Analysis program. According to Section 4.3.1 of the FDOT Drainage Manual, the design frequency of the cross drain is 50 years. The Pond Siting Report, dated 4/2011, established a contributing drainage area of 302.1 acres upstream of the cross drain. This area was used in conjunction with hydraulic data provided in the Drainage Complaint Report, dated 1/2010, to calculate an expected 50-year discharge of 145.0 cfs through the cross drain.

After the expected discharge was determined, site data pulled from the As-Builts for the Drainage Improvement Project completed in 2013 was used to model the triple 30" cross drain in HY-8. As can be seen in **Figure 16**, the hydraulic model anticipates significant overtopping for the 50-year design frequency. See **Appendix K** for the hydraulic modeling information and results.



Figure 16: Existing Cross Drain Model

Verifying that the current capacity of the triple 30" cross drain was insufficient to accommodate the 50year discharge, a proposed model with three 42" pipes was generated to evaluate the impact increasing the diameter of the pipes would have on the cross-drain capacity. Although upsizing of the pipes did prove to reduce the headwater elevation by about 0.5-ft, the expected water surface elevation was still about 0.5-ft above the crown of the road. See **Figure 17** for a graphical representation of the proposed triple 42" cross drain.







Figure 17: 42" Pipe Cross Drain Model

With this information, a second proposed model was then generated in which the roadway profile was raised 2-ft. Raising the profile in conjunction with keeping the pipes at a 42" diameter resulted in a less significant reduction in the headwater elevation; however, it also put the water surface elevation roughly 0.75-ft below the edge of shoulder, demonstrating no anticipated overtopping for the 50-year event. See **Figure 18** for a graphical representation of the proposed triple 42" cross drain with 2-ft profile raise.



Figure 18: Proposed 42" Pipe and Roadway Profile Raise Cross Drain Model



# 5.0 RECOMMENDED COUNTERMEASURES

#### Drainage:

- Raise the profile of SR 29 by fully reconstructing between MP 4.8 to MP 6.5 to provide a permanent solution to the roadway overtopping issues.
- Increase the size of the cross drain at MP 5.528 to a minimum of quadruple 42" culverts to be able to handle the flow of a 50-year storm event.
- Raise existing bridge profiles to expand the hydraulic capabilities and provide 2' drift clearance.
- Provide a treatment and attenuation swale to formally treat the roadway runoff prior to discharge off-site.

#### Wildlife Crossings and Fencing:

- Provide wildlife crossing opportunities along the corridor by reconstructing the bridges to include wildlife crossing features (shelves). This will help minimize the risk of animal-vehicle collisions which are the highest crash type by percentage.
- Provide wildlife fencing along the corridor to help funnel wildlife to the reconstructed bridges and culverts which will be improved to provide wildlife crossing features. This will help create a barrier to keep wildlife off the roadway and away from oncoming vehicle traffic.

#### <u>Safety</u>

- Provide nighttime lighting at the intersections with SR 78 and CR 74. This can help to eliminate nighttime crashes at these intersections and help prevent drivers from not seeing the upcoming STOP Signs and oncoming traffic.
- Provide ground in rumble strips on the centerline throughout the entire corridor. This will help to alert drivers of when they are veering into oncoming traffic. All 5 fatal crashes in the corridor were head on collisions.
- Provide ground in rumble strips on the outside shoulders. This can help to prevent drivers from running off the road. Run off road crashes were the third highest crash type in this corridor.

#### **Structures**

• Replace all four bridges in the study segment since they have exceeded their design life by over 24 years. The new bridges will be longer and have more vertical clearance to improve hydraulics and wildlife connectivity.

#### <u>Roadway</u>

- Resurface from MP 1.85 to MP 12.393 (except full reconstruction between MP 4.8 to MP 6.5) to eliminate the deficient crack rating.
- Provide new signing and pavement markings from MP 1.85 to MP 12.393. This will help to improve the reflectivity in the corridor which was seen to have the lowest rating for visibility from the most recent survey in 2021. This can also help to reduce the nighttime crashes by increasing driver visibility.



# 6.0 CONCLUSION AND RECOMMENDATIONS

Overall, this corridor has multiple safety issues related to existing crash types and drainage conditions. It is recommended that a reconstruction project be programmed to raise the profile of the roadway to eliminate the flooding issues between MP 4.8 and MP 6.5. Additionally, this project includes resurfacing the remaining portion beginning at MP 1.85 and ending at MP 12.393 to improve safety and fulfill maintenance needs. New signing and pavement markings should also be part of this project. The project should include the replacement of the aging bridges and incorporate wildlife crossing features including wildlife fencing. Preventing roadway closures on SR 29 in the future is critical for freight travel in the region as the nearest detour to get from Labelle to US 27 is over 25 miles in length. See **Appendix L** for detour map.



Figure 19: Recommended Project Limits, Bridge Replacements and Cross Drain Replacements

# APPENDIX A Straight-Line Diagram

		5 YR INV	SLD REV	BMP EMP	NV	SLD REV	19 <i>0</i> 1	
DATE		10/31/2019	11/21/2019				FLORIDA DEPARTMENT OF TRANSPORTATION	SECTION STATUS INT. or US ROUTE NO. ST
PV .		CTC	стс				STRAIGHT LINE DIAGRAM OF ROAD INVENTO	$\mathbf{PRY}$ 02
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		m 58.0' - 24.	0'					ð
		_ 2 - 12.0' R	RDWY					
	ROADWAY	2 - 5.0' PV	/D SHLD1					$z \stackrel{\sim}{O} \bigotimes_{n=1}^{58.0'} 24.0'$
	FEATURES	2 - 12.0' L	WN SHLD2					
		70.0' - 24.0'	'0.0' - 24.0'					2 12 0'L WN SHLD2
		2 - 12.0' RDWY 💦 2	2 - 12.0' RDWY 58.0' - 24.0'					m 45.0' - 24.0'
	LANE WIDTHS	12.0 PVD MED _ 1	2.0 PVD MED 8 2 12.0' RDWY					∞ 2 - 12.0' RDWY
	THE ATENNOLD	2 - 5.0' PVD SHLD1 22	2 - 5 0' PVD SHLD1 0 2 - 5 0' PVD SHL	01				- 10.0' PVD SHLD1 - LT
_		2 - 12.0" LWIN SHLD2 2	2 12 0 LWN SHLD2 2 12 0 LWN SHI	LD2				11.0 PVD SHLD1 - R1
	ROADWAY	28/FC-4 28/FC-4 2	8/FC-428/FC-2					
	ROADWAT	113	364					
C	OMPOSITION	28/FC-4 0/2	8/FC-4					
		∆=38°04'00.00" CURVE						
	IORIZONTAL	P=2°00'00 00 FIELD \	VERIFIED					
- I '		PC=0.000						
	ALIGNMENT	PI=0.025 PT=0.129						
		B=N37°5	51'00"E					
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	RE AVERAGED							
								52.0' - 26.0' 58.0' - 24.0'
		58 0' - 24 0'					Q 45 0' - 24	
		2 12 0' RDWY						3000000000000000000000000000000000000
		2 - 5.0' PVD SHLD1						SHLD1 LT 4.0'LWN SHLD2 LT 12.0'LWN SHLD2 LT
		2 - 12.0' LWN SHLD2					' 11.0' PVE	SHLD1 - RT 12.0' LWN SHLD2 - RT 12.0' LWN SHLD2 - RT
	ROADWAY	28/FC-4						
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Version: 1.4.2.27 11/21/2019



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LANE WIDTHS ARE AVERAGED	58 0' - 24 0'					
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ROADWAY						
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				<b>X</b> 62.0' - 24.0' <b>O</b> 2 - 12.0' RDWY		0 2 - 12.0' RDWY 0 2 2 12.0 PVD MED 0 2 2
				Q 2 - 7.0 PVD SHLD1		← 2 5.0' PVD SHLD1 ← 2 3.0' LWN SHLD2 ← 3.0' LWN SHLD2
ARE AVERAGE						← 62.0' - 12.0'L+1
	58.0' - 24.0'			<b>80</b> 58.0° - 24.0° <b>60</b> 2- 12.0° <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0°</b> <b>71.0</b>	12.0'L+13.0'R 0'L + 1 - 13.0'R RDWY 858.0' - 2	4.0' 0 12.0 PVD MED
	2 - 12.0' RDWY 2 - 5.0' PVD SHLD1 2 - 12.0' LWN SHLD2			00         44.0         ∠4.0         12.0 F           00         2         12.0 RDWY         ∠         2.5.0           02         10.0 RVD         ∠         2.5.0           02         10.0 RVD         ∠         2.5.0	VU MED         ► 2 - 12.0'           )' PVD SHLD1         ► 2 - 5.0' F           0' UWN SHLD2         ► 2 - 5.0' F	KDWY         ↓ 2 - 5.0' PVD SH           PVD SHLD1         3.0' LWN SHLD2
ROADWAY	28/FC-4			- 2 - 10.0 PVD SHLU1 2-12	4 28/FC-4	28/FC-4 - 22
COMPOSITIO	N					
	CURVE DATA NOT	FIELD VERIFIED		25/10		11.767
HORIZONTA	-				PI=11 PT=1	I.955 (2.139
ALIGNMENT					∆=19 D=1°	1 <sup>°</sup> 38'30 <u>.00</u> '' 00'00 00
				0.033		
STRUCTURE						
DESCRIPTIO	N					
DISTRICT US	E					
SIS	CORRIDOR					
FUN CLASS	RURAL PRIN ART OTHER					

Version: 1.4.2.27 11/21/2019



# APPENDIX B Evacuation Route Map



# **APPENDIX C** Intersection Photos

#### SR 29 & SR 78 Intersection



SR 29 Northbound View



SR 29 Southbound View



SR 78 Westbound View



### SR 29 Northbound View



#### SR 29 Southbound View



CR 74 Eastbound View

# APPENDIX D Reflectivity Ratings 2021

Rdyld	Report Group	BM P	EM	Date	Туре	LEL	L5S	L4SL	L3S	L2S	L1S	LCL	RCL	R1S	R2S	R3S	R4S	R5S	REL	SpdL mt	AAD T	Surface
05090000	2022 Survey	0.0	0.1	2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	187	0	0	0	0	0	0	50	6100	FRICTION 12.5
05090000	2022 Survey	0.1	0.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	187	0	0	0	0	0	0	50	6100	FRICTION 12.5
05090000	2022 Survey	0.2	0.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	167	0	0	0	0	0	0	50	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.3	0.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	172	0	0	0	0	0	0	50	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.4	0.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	190	0	0	0	0	0	0	50	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.5	0.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	195	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.6	0.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	189	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.7	0.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	170	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.8	0.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	189	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	0.9	1.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	163	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.0	1.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	153	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.1	1.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	169	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.2	1.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	212	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.3	1.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	214	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.4	1.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	176	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.5	1.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	185	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.6	1.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	209	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.7	1.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	195	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.8	1.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	170	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	1.9	2.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	171	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.0	2.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	134	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.1	2.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	187	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.2	2.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	177	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.3	2.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	217	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.4	2.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	185	0	0	0	0	0	0	60	6100	FRICTION 12.5 MOD
05090000	2022 Survey	2.5	2.6	2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	183	0	0	0	0	0	0	60	6100	FRICTION 12.5

Rdyld	Report Group	BM	EM	Date	Туре	LEL	L5S	L4SL	L3S	L2S	L1S	LCL	RCL	R1S	R2S	R3S	R4S	R5S	REL	SpdL	AAD	Surface
				00:00			╎┖		╎┖	╎┖	╎┖			•								Туре
05090000	2022 Survey	2.6	2.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	205	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	2.7	2.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	130	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	2.8	2.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	156	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	2.9	3.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	128	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.0	3.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	122	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.1	3.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	114	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.2	3.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	118	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.3	3.4			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05090000	2022 Survey	3.4	3.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	124	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.5	3.6			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05090000	2022 Survey	3.6	3.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	109	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.7	3.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	100	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.8	3.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	132	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	3.9	4.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	145	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.0	4.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	99	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.1	4.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	110	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.2	4.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	85	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.3	4.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	115	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.4	4.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	101	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.5	4.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	100	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.6	4.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	137	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.7	4.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	128	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.8	4.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	94	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	4.9	5.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	104	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.0	5.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	145	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.1	5.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	111	0	0	0	0	0	0	60	3600	FRICTION 12.5

Rdyld	Report Group	BM P	EM P	Date	Туре	LEL	L5S	L4SL	L3S	L2S	L1S	LCL	RCL	R1S	R2S	R3S	R4S	R5S	REL	SpdL mt	AAD T	Surface
05090000	2022 Survey	5.2	5.3	2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	132	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.3	5.4	2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	134	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.4	5.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	129	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.5	5.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.6	5.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	118	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.7	5.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	126	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.8	5.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	140	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	5.9	6.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	122	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.0	6.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	131	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.1	6.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	120	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.2	6.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	101	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.3	6.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	107	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.4	6.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	126	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.5	6.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	60	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.6	6.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	111	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.7	6.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	131	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.8	6.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	146	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	6.9	7.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	135	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.0	7.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	159	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.1	7.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	153	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.2	7.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	147	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.3	7.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	141	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.4	7.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	139	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.5	7.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	138	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.6	7.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	124	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.7	7.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	111	0	0	0	0	0	0	60	3600	FRICTION 12.5

Rdyld	Report Group	BM P	EM P	Date	Туре	LEL	L5S	L4SL	L3S	L2S	L1S	LCL	RCL	R1S	R2S	R3S	R4S	R5S	REL	SpdL	AAD T	Surface
05090000	2022 Survey	7.8	7.9	2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	7.9	8.0	00:00 2021-06-04T00:	INVENTORY	0	0	0	0	0	0	0	134	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.0	0 1	00:00 2021-06-04T00:		0	0	0	0	0	0	0	142	0	0	0	0	0	0	60	2600	EDICTION 12.5
03090000	2022 301769	0.0	0.1	00:00	INVENTORI		0	0	0				142	0	0	0	0			00	3000	TRICTION 12.5
05090000	2022 Survey	8.1	8.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.2	8.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	131	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.3	8.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	160	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.4	8.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	146	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.5	8.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	123	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.6	8.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	143	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.7	8.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	141	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.8	8.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	144	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	8.9	9.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	126	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.0	9.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	138	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.1	9.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	140	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.2	9.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	111	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.3	9.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.4	9.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	120	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.5	9.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	146	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.6	9.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	132	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.7	9.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	113	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.8	9.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	135	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	9.9	10.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	120	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.0	10.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.1	10.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	126	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.2	10.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	135	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.3	10.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	149	0	0	0	0	0	0	60	3600	FRICTION 12.5

Rdyld	Report Group	BM	EM	Date	Туре	LEL	L5S	L4SL	L3S	L2S	L1S	LCL	RCL	R1S	R2S	R3S	R4S	R5S	REL	SpdL	AAD	Surface
		P	P				L		L	L	L			L	L	L	L	L		mt	T	Туре
05090000	2022 Survey	10.4	10.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	131	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.5	10.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	141	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.6	10.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	136	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.7	10.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	144	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.8	10.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	135	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	10.9	11.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	112	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.0	11.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	142	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.1	11.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	157	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.2	11.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	157	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.3	11.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	110	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.4	11.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	202	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.5	11.6	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	170	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.6	11.7	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	184	0	0	0	0	0	0	60	3600	FRICTION 12.5
05090000	2022 Survey	11.7	11.8	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	180	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	11.8	11.9	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	201	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	11.9	12.0	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	185	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	12.0	12.1	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	189	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	12.1	12.2	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	163	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	12.2	12.3	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	148	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	12.3	12.4	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	190	0	0	0	0	0	0	60	4600	FRICTION 12.5
05090000	2022 Survey	12.4	12.5	2021-06-04T00: 00:00	INVENTORY	0	0	0	0	0	0	0	188	0	0	0	0	0	0	60	4600	FRICTION 12.5
# APPENDIX E Skid Ratings Report 2020

Project Id Dist	trict Id County Name	System State Highway Name	Collected Date	Roadway Id Direction	Lane Number	Milepost	Corrected Friction Number	Friction Number Average	Friction Number Standard Deviation	Speed Average	Mean Profile Depth	Percent Error	Latitude	Longitude
722	1 Glades	1 29	1/6/2020	5090000 L	1	0.366	46	46	1.025	40.6	0.05	2.38	26.788135	-81.43386
722	1 Glades	1 29	1/6/2020	5090000 L	1	0.668	47	47	1.811	40.6	0.057	4.78	26.791605	-81.430868
722	1 Glades	1 29	1/6/2020	5090000 L	1	0.979	47	47	0.568	40.4	0.043	2.09	26.795182	-81.427772
722	1 Glades	1 29	1/6/2020	5090000 L	1	1.424	44	44	0.72	40.5	0.046	1.87	26.800282	-81.423368
722	1 Glades	1 29	1/6/2020	5090000 L	1	1.674	48	48	0.971	40.4	0.044	1.98	26.803145	-81.420892
722	1 Glades	1 29	1/6/2020	5090000 L	1	1.967	45	45	0.761	40.4	0.046	2.8	26.806517	-81.417977
722	1 Glades	1 29	1/6/2020	5090000 L	1	2.349	45	45	0.772	40.4	0.047	2.13	26.810896	-81.414196
722	1 Glades	1 29	1/6/2020	5090000 L	1	2.555	47	47	1.094	40.4	0.038	2.13	26.813274	-81.412161
722	1 Glades	1 29	1/6/2020	5090000 L	1	2.955	45	45	0.776	40.2	0.028	1.33	26.817856	-81.408185
722	1 Glades	1 29	1/6/2020	5090000 L	1	3.329	44	44	0.952	40.4	0.027	1.34	26.822153	-81.40447
722	1 Glades	1 29	1/6/2020	5090000 L	1	3.673	43	43	1.097	40.4	0.028	1.77	26.826102	-81.401058
722	1 Glades	1 29	1/6/2020	5090000 L	1	3.973	43	43	0.75	40.4	0.029	1.3	26.829553	-81.39808
722	1 Glades	1 29	1/6/2020	5090000 L	1	4.376	43	43	1.357	40.5	0.028	1.46	26.834179	-81.394085
722	1 Glades	1 29	1/6/2020	5090000 L	1	4.656	45	45	0.791	41.1	0.028	1.96	26.83739	-81.391312
722	1 Glades	1 29	1/6/2020	5090000 L	1	5.125	45	45	0.822	40.2	0.027	1.43	26.842764	-81.386664
722	1 Glades	1 29	1/6/2020	5090000 L	1	5.367	44	44	0.595	40.5	0.029	1.69	26.845543	-81.384263
722	1 Glades	1 29	1/6/2020	5090000 L	1	5.671	42	42	0.551	40.3	0.026	1.51	26.849031	-81.381245
722	1 Glades	1 29	1/6/2020	5090000 L	1	6.044	42	42	0.853	40.7	0.028	1.4	26.85332	-81.37754
722	1 Glades	1 29	1/6/2020	5090000 L	1	6.372	43	43	1.22	40.4	0.028	1.58	26.857083	-81.374287
722	1 Glades	1 29	1/6/2020	5090000 L	1	6.653	42	42	0.872	40.4	0.027	1.64	26.860317	-81.371489
722	1 Glades	1 29	1/6/2020	5090000 L	1	6.976	42	42	0.433	40.4	0.029	1.43	26.864021	-81.36829
722	1 Glades	1 29	1/6/2020	5090000 L	1	7.38	44	44	0.715	40.3	0.028	1.83	26.868662	-81.364275
722	1 Glades	1 29	1/6/2020	5090000 L	1	7.653	48	48	0.607	40.2	0.035	1.77	26.871795	-81.361568
722	1 Glades	1 29	1/6/2020	5090000 L	1	7.989	45	45	0.925	40.3	0.03	1.46	26.875652	-81.358233
722	1 Glades	1 29	1/6/2020	5090000 L	1	8.438	43	43	0.61	40.4	0.027	1.27	26.880802	-81.353782
722	1 Glades	1 29	1/6/2020	5090000 L	1	8.668	45	45	0.551	40.4	0.028	1.71	26.883445	-81.351494
722	1 Glades	1 29	1/6/2020	5090000 L	1	8.893	45	45	1.318	40.7	0.027	1.41	26.886038	-81.349255
722	1 Glades	1 29	1/6/2020	5090000 L	1	9.386	44	44	0.833	40.4	0.029	2.04	26.891692	-81.344365
722	1 Glades	1 29	1/6/2020	5090000 L	1	9.678	44	44	0.931	40.6	0.027	1.57	26.895049	-81.341459
722	1 Glades	1 29	1/6/2020	5090000 L	1	9.978	45	45	0.583	40.3	0.029	1.9	26.898494	-81.338481
722	1 Glades	1 29	1/6/2020	5090000 L	1	10.421	43	43	0.931	40.4	0.03	2.13	26.903582	-81.334083
722	1 Glades	1 29	1/6/2020	5090000 L	1	10.678	42	42	0.762	40.3	0.026	1.37	26.906527	-81.331529
722	1 Glades	1 29	1/6/2020	5090000 L	1	10.977	43	43	0.824	40.5	0.03	2.67	26.909972	-81.328549
722	1 Glades	1 29	1/6/2020	5090000 L	1	11.275	46	46	0.963	41.2	0.029	1.09	26.913397	-81.325591
722	1 Glades	1 29	1/6/2020	5090000 L	1	11.612	42	42	0.578	40.4	0.037	2.11	26.917272	-81.322255
722	1 Glades	1 29	1/6/2020	5090000 L	1	11.929	39	39	0.63	40.5	0.035	1.22	26.920776	-81.318923
722	1 Glades	1 29	1/6/2020	5090000 L	1	12.216	40	40	0.862	40.1	0.031	1.31	26.923209	-81.315154

# APPENDIX F Crash Reports

24400691	3/16/2021	12:00 AM	26 91215313	-81.32661596	Clear	Dry	Animal		NIGHT
24400699	3/28/2021	6:00 AM	26.91800828	-81.32141837	Fog, Smog, Smoke	Unknown	Off Road	No Injury	NIGHT
24400703	5/6/2021	5:45 AM	26.90056999	-81.33665009	Fog, Smog, Smoke	Dry	Head On	No Injury	NIGHT
24400804	10/3/2021	11:25 PM	26.91787374	-81.32153927	Clear	Dry	Off Road	No Injury	NIGHT
24400847	11/21/2021	6:00 AM	26.91791673	-81.32151984	Cloudy	Wet	Off Road	No Injury	NIGHT
24400858	12/ //2021	2:55 AM	26.836/0189	-81.391/4594	Fog, Smog, Smoke	Dry	Animal Hood On	No Injury Rescible Injury	
24400803	1/3/2022	3:00 PM	26.81250225	-81.41239202	Clear	Dry	Angle	No Injury	DAY
24400882	1/20/2022	7:31 AM	26.83512537	-81.39329272	Clear	Dry	Animal	No Injury	DAY
82063892	2/2/2016	10:10 AM	26.92353948	-81.3145142	Cloudy	Dry	Sideswipe	No Injury	DAY
82770968	11/22/2016	7:19 PM	26.81251571	-81.41272931	Clear	Dry	Other	No Injury	NIGHT
82771016	6/13/2017	5:00 AM	26.91788999	-81.32165	Clear	Dry	Other	No Injury	NIGHT
827/1094	8/30/2017	9:00 PM	26.86207555	-81.36983426	Rain	Wet	Off Road Sidoswino	No Injury	NIGHI
82771126	1/3/2018	9.25 PIVI 3.27 PM	26.81247999	-81.41270	Clear	Dry	Other	No Injury	
82771243	1/14/2017	4:38 AM	26.81247999	-81.41276	Clear	Dry	Animal	No Injury	NIGHT
82771435	4/7/2018	3:00 PM	26.9247908	-81.31230536	OTHER	Unknown	Other	UNKNOWN/NOT CODED	UNKNOWN
82771535	2/11/2020	6:14 AM	26.86947851	-81.3634045	Fog, Smog, Smoke	Dry	Animal	No Injury	NIGHT
82771592	9/18/2016	4:50 AM	26.86414653	-81.36828946	Rain	Wet	Other	Possible Injury	NIGHT
82771623	12/15/2015	6:20 AM	26.82000616	-81.40621345	Fog, Smog, Smoke	Dry	Sideswipe	No Injury	NIGHT
827/1659	1/10/2017	7:55 PM	26.82396006	-81.40286181	Clear	Dry	Off Road	Incapacitating Injury	NIGHI
82771691	12/28/2020	3:03 AM	26.83335262	-81.39484848	Clear	Dry	Animal	No Injury	NIGHT
83786691	10/3/2015	10:40 PM	26.91215313	-81.32661596	Clear	Dry	Animal	Possible Injury	NIGHT
83797736	6/24/2015	10:11 PM	26.81247996	-81.41275998	Clear	Water (standing/moving)	Off Road	Possible Injury	NIGHT
84517286	1/3/2016	9:58 AM	26.81247999	-81.41276	Cloudy	Dry	Rear End	Possible Injury	DAY
84543075	1/21/2016	7:11 AM	26.86440999	-81.36793	Clear	Dry	Other	No Injury	DAY
85146043	9/9/2016	10:08 AM	26.92062276	-81.31900026	Clear	Dry	Rollover	Non-Incapacitating Injury	DAY
85185004	11/26/2015	9:30 PM	26.86440999	-81.36793	Clear	Dry	Animal	No Injury	NIGHT
85192700	2/2//2016	10.37 PIVI A:35 PM	26.81247999	-81.41270	Bain	Dry Wet	Unknown	Fatal (within 30 days)	
85192768	8/19/2016	3:30 PM	26.91788999	-81.32165	Clear	Drv	Left Turn	No Injury	DAY
85208851	11/20/2016	2:15 PM	26.89216696	-81.34392473	Clear	Dry	Animal	Possible Injury	DAY
85258238	3/8/2016	10:25 AM	26.91793296	-81.3216133	Clear	Dry	Angle	No Injury	DAY
85258267	7/7/2016	11:52 AM	26.85429815	-81.37663093	Clear	Dry	Other	No Injury	DAY
85294447	12/28/2016	6:29 AM	26.8083607	-81.41633847	Clear	Dry	Rollover	Incapacitating Injury	NIGHT
85294448	1/9/2017	4:47 AM	26.81247999	-81.41276	Clear	Dry	Animal	No Injury	NIGHT
85326204	8/22/2016	5:10 PM	26.81247999	-81.41276	Clear	Dry	Rear End	Incapacitating Injury	DAY
85373095	6/12/2017	5:45 AM	26.91788999	-81.32165	Clear	Wet	Other	No Injury	
85395230	11/6/2016	8:09 PM	26.91788999	-81.32165	Clear	Dry	Animal	No Injury	NIGHT
85455680	3/8/2017	6:58 PM	26.91788999	-81.32165	Clear	Dry	Animal	No Injury	DAY
85455694	4/22/2017	10:30 PM	26.82396006	-81.40286181	Clear	Dry	Animal	No Injury	NIGHT
85455740	12/28/2017	1:53 PM	26.87580488	-81.35807408	Clear	Dry	Other	Fatal (within 30 days)	DAY
85463418	8/4/2017	6:39 AM	26.80904744	-81.41575055	Cloudy	Wet	Other	Incapacitating Injury	NIGHT
85473996	11/4/2017	7:55 AM	26.91788999	-81.32165	Clear	Dry	Off Road	No Injury	DAY
85475015	12/3/2017	11:32 AM	26.91788999	-81.32165	Clear	Dry	Right Turn	No Injury	DAY
85499758	2/5/2017	4:05 AIVI	26.91788999	-81.32105	Clear Fog Smog Smoke	Dry	Rollover	No injury	
85519391	4/6/2018	7:50 PM	26.91788999	-81.32165	Clear	Dry	Off Road		NIGHT
85560414	3/31/2018	10:26 PM	26.8122576	-81.41294522	Clear	Dry	Off Road	Incapacitating Injury	NIGHT
85561725	6/15/2018	9:32 AM	26.91788999	-81.32165	Clear	Dry	Rollover	No Injury	DAY
85573513	7/31/2018	5:25 AM	26.80683926	-81.41764257	Clear	Dry	Animal	POSSIBLE INJURY	NIGHT
85586995	8/27/2018	8:13 AM	26.86440999	-81.36793	Clear	Dry	Rollover	No Injury	DAY
85605345	11/22/2017	5:35 AM	26.87580763	-81.35803572	Fog, Smog, Smoke	Dry	Head On	Fatal (within 30 days)	NIGHT
8/104488	11/28/2019	10:30 AM	26.91788999	-81.32165	Clear	Dry	Off Bood	No Injury	DAY
87140425	5/27/2018	8.10 PIVI 11:44 PM	26.91788999	-81.32105	Clear	Dry	Animal	No Injury	NIGHT
87140893	3/30/2019	10:59 PM	26.81247999	-81.41276	Clear	Dry	Left Turn	Possible Injury	NIGHT
87168190	5/12/2019	1:40 PM	26.81247999	-81.41276	Rain	Wet	Animal	No Injury	DAY
87177808	7/3/2018	3:11 PM	26.85868231	-81.37284778	Rain	Wet	Head On	Fatal (within 30 days)	DAY
87204132	10/12/2018	10:00 PM	26.81247999	-81.41276	Clear	Dry	Left Turn	Incapacitating Injury	NIGHT
87206611	9/24/2019	6:04 AM	26.81247999	-81.41276	Clear	Dry	Off Road	No Injury	NIGHT
87209220	//28/2018	4:41 PM	26.81247596	-81.41266941	Rain	Wet	Rear End	No Injury	
87281333	4/27/2018	12:25 AM	26.91788999	-81.41270	Clear	Dry	Animal	No Injury	NIGHT
87920374	6/8/2018	12:56 AM	26.83571758	-81.39278983	Rain	Wet	Other	No Injury	NIGHT
87920393	7/21/2018	5:16 AM	26.91788999	-81.32165	Clear	Dry	Off Road	No Injury	NIGHT
87920408	8/24/2018	12:10 PM	26.83387669	-81.39439494	Clear	Dry	Other	No Injury	DAY
87920428	10/31/2018	1:30 AM	26.9180853	-81.32140398	Clear	Dry	Off Road	No Injury	NIGHT
87920439	9/29/2018	11:15 PM	26.91788999	-81.32165	Clear	Dry	Other	Possible Injury	NIGHT
87920440	2/0/2010	2:30 AM	26.86830854	-81.36461402	Clear	Dry	Uther	Possible Injury	
87920408	2/27/2019	9:43 PM	26.81011284	-81.41491917	Clear	Drv	Animal	No Injury	NIGHT
87920498	2/21/2019	8:10 AM	26.81247999	-81.41276	Clear	Dry	Other	No Injury	DAY
87920546	4/9/2021	9:27 AM	26.91826763	-81.32142975	Clear	Dry	Left Turn	No Injury	DAY
87920564	8/18/2019	4:00 AM	26.91801968	-81.3214295	Clear	Dry	Other	No Injury	NIGHT
87920587	8/12/2019	12:54 AM	26.88279701	-81.3519045	Clear	Dry	Animal	No Injury	NIGHT
87920589	9/8/2019	3:20 AM	26.81161796	-81.41352257	Clear	Dry	Other Off Bood	No Injury	NIGHT
82215220	4/29/2021 9///2020	10.50 PIVI	20.01248001	-01.412/b	Cloudy	Dry	Officer	No Injury	
88215227	10/21/2020	5:30 AM	26.81248001	-81.41276	Clear	Drv	Rear End	No Iniurv	NIGHT
88337366	4/20/2020	10:25 PM	26.84968689	-81.38045172	Cloudy	Wet	Other	No Injury	NIGHT
88337373	6/25/2020	1:55 AM	26.81249149	-81.41277034	Clear	Dry	Off Road	No Injury	NIGHT
88337395	11/2/2020	8:45 PM	26.81248359	-81.41276679	Clear	Dry	Off Road	No Injury	NIGHT
88337409	1/6/2021	7:45 PM	26.81247998	-81.41273892	Clear	Dry	Off Road	No Injury	NIGHT
88338261	6/25/2020	3:40 PM	26.90863859	-81.32964354	CLOUDY	Dry	Rollover	NU-INCAPACITATING INJURY	
88428603	1/27/2021	2.13 AIVI 4.30 PM	20.00222682	-81 32165005			Sideswine	Incanacitating Injury	
88436409	2/5/2022	12:05 AM	26.81248001	-81.41276	Clear	Dry	Angle	Incapacitating Injury	NIGHT
89452577	10/29/2019	6:40 AM	26.89373601	-81.34238917	Fog, Smog, Smoke	Dry	Animal	No Injury	NIGHT
89452586	11/26/2019	6:10 PM	26.82396006	-81.40286181	Clear	Dry	Animal	No Injury	DAY
89452613	1/16/2020	7:08 PM	26.86767651	-81.36523167	Clear	Dry	Animal	No Injury	NIGHT
89452657	8/24/2020	6:45 AM	26.81180639	-81.41322067	Clear	Dry	Animal	No Injury	NIGHT
89452664	5/10/2020	1:15 PM	26.90235411	-81.33516522	Rain	Wet	Off Road	Possible Injury	
89452679	4/30/2020	4.55 AIVI	20.91789	-81 32165005	Rain	Dry Met	Off Road	No Injury	
89452684	5/12/2020	3:33 PM	26.91789	-81.32165005	Clear	Dry	Other	No Injury	DAY
89452711	8/27/2020	5:00 PM	26.85032578	-81.38003243	Clear	Dry	Other	No Injury	DAY
89452720	11/26/2020	11:10 PM	26.81258407	-81.41280536	Clear	Dry	Other	No Injury	NIGHT
89452724	11/5/2020	5:45 AM	26.85749985	-81.37399179	Clear	Dry	Animal	No Injury	NIGHT
89452738	1/20/2021	10:25 PM	26.86419495	-81.36811318	Clear	Dry	Animal	No Injury	NIGHT
89452768	9/26/2020	8:20 AM	26.86435696	-81.36788291	Clear	Dry	Animal	No Injury	DAY
09452//U 89452781	11/30/2020	11.55 AIVI	20.91/89	-01.32105005	Rain	Ury Wot	Animai Left Turo	No Injury	υΑτ ΠΔΥ
89452787	3/1/2021	4:30 PM	26.82742996	-81.39979673	Clear	Drv	Sideswipe	No Injury	DAY
89452791	2/27/2021	11:15 PM	26.81234698	-81.41298877	Fog, Smog, Smoke	Dry	Animal	No Injury	NIGHT
89452809	1/13/2021	8:13 PM	26.91789	-81.32165005	Rain	Wet	Animal	No Injury	NIGHT

Crash not in study area

FLORIDA	TRAFFIC CF	RASH REPORT
	SHORT FORM	UPDATE

#### HIGHWAY SAFETY & MOTOR VEHICLES, TRAFFIC CRASH RECORDS NEIL KIRKMAN BUILDING, TALLAHASSEE, FL 32399-0537

	(E	lectronic Versior	n)				01201104, 174			
Date of Crash 07/Nov/2015 04:35 I	PM Time of 0 07/Nov	Crash #/ <b>2015 04:35 PM</b>	Date of Report 07/Nov/2015 05:29 Pl	Invest. Age	ncy Report Nun FHPF15OFF	1ber <b>-069783</b>	HSM	/ Crash Report 8	Number 35192700	
CRASH IDENTIFIE	RS									
County Code City 60	Code 0	County of Crash G	BLADES	Place or City o UN	of Crash INCORPORAT	ED	Within City Lim No	its Time R 07/N 04:	eported ov/2015 35 PM	Time Dispatched 07/Nov/2015 04:39 PM
Time on Scene Time 07/Nov/2015 07 04:47 PM	e Cleared Sce /Nov/2015 08 PM	ne Completed :30 Yes	Reason (if Investigation	NOT Complete	ed)			No	tified By Law En	forcement
ROADWAY INFOR	MATION							2		
Crash Occured On Str	eet, Road, Hiç	jhway SR-29		0	At Street Addr	ess#	O At 26.8	Lattitude 112117947138	and 1 198 -81.41	_ongitude  3776695766899
At Feet Or M	Ailes	Direction South	OFrom Intersection Wit	h Street, Road	, Highway <b>SR-78</b>		5	0.0	🗘 Or Fro	m Milepost #
Road System Identifier	3 State		Type Of Shoulder	1 Pave	d		ype Of Intersec	tion 1 Not at Inte	ersection	
	ION (Check	t if Pictures Tal	ken)							
light Condition 1 Daylight	Wea	ther Condition 3 Rain	Roadway Surfa	ace Condition Wet	School Bus F	Related 1 No		Manner Of Co	ollision <b>3 Anale</b>	9
First Harmful Event Ty	pe	First Harmful Ev	rent Fi 14	rst Harmful Eve <b>1 On</b>	ent Location Roadway	Within	Interchange No	L First Harmful Ev <b>1</b>	vent Relatio Non.Junct	n to Junction ion
Contributing Circumsta	ances: Road		Contributing Circu	mstances: Roa		4	Contributing Circ	umstances: Ro	ad	
Contributing Circumsta	ances: Enviror	ment	Contributing Circu	mstances: Envi	ronment		Contributing Circ	umstances: En	vironment	
Work Zone Related 1 No	Crash In Wor	k Zone	Type Of	Work Zone		Worl	kers In Work Zo	ne Law Enf	orcement Ir	n Work Zone
VEHICLE (Check if	Commercia	al) 🗌		0	<del>x</del>		)			
Vehicle Motor Vehicle 2 1 Vehicle in	e Type n <b>Transport</b>	Hit and Run 1 No	Veh License Number	State	Reg. Exp 14/Ju	oires Per n/2016	manent Reg. \ No	/IN		
Year Make Mi 2004 DODG D	odel Style	PK Color	Extent of Damage Disabling	Est. Dam 100	age Towed	Due To Dam Yes	age Vehicle TIMS	Removed By <b>TOWING (863</b>	B)675	tation Rotation
Insurance Company				In	surance Policy I	Number				
Name of Vehicle Owne	er (Check Box	If Business)	Current Add	dress (Number	and Street)		City	and State		Zip Code
Trailer License Num One:	ber Stat	e Reg. Expire	es Permanent Reg. V	/IN			Year	Make	Length	Axles
Trailer License Num Two:	ber Stat	e Reg. Expire	es Permanent Reg. V	/IN			Year	Make	Length	Axles
Traveling: Direction	On Stree	t, Road, Highway		SR-29			ATE	55 st. Speed	isted Speed 60	l otal Lanes 2
CMV Configuration	•		Cargo Body Type			Area of	Initial Impact	M	ost Damage	d Area
Comm GVWR/GCWR		Tra	iler Type (trailer one)	Trailer Type	trailer two)	2 3 4 5	8 7 18. Under	carriage	4 5 8 7 18 17 8	<ul> <li>18. Undercarriage</li> <li>19. Overturn</li> </ul>
Haz. Mat. Release	Haz Mat. Plac	ard Number	CI	ass		14 13 12 11	10 8 21. Traile	ar 14 13	12 11 10 0	20. Windshield 21. Trailer
Motor Carrier Name		, i	US D	OT Number						
	Motor Carrier	Address			City and Sta	ate		Zip Cod	le P	hone Number
Comm/Non-Commerci	al Vehicle E	Body Type 3 Pickup	Vehicle Defects (one 1 None	e) 9	Vehicle Defects	(two)	Emergen	cy Vehicle Use <b>1 No</b>	Speciual 1 No Sp	Function of MV pecial Function
Vehicle Maneuver Acti 1 Straight Ahead	on Trafficwa 1 Two	iy Way, Not Divide	d Roadway Grade <b>1 Level</b>	Roadwa	ay Alignment 1 Straight	Most Har 2 Coll	mful Évent lision with Non Object	-Fixed Most	Harmful Ev Aotor Vehic	ent Detail cle in Transport
Traffic Control Device 1 No Con	For This Vehic trols	tle First (1) Seque	ence of Events Se with Non-Fixed	cond (2) Seque	ence of Events	Third (3) S	equence of Eve	nts Fourth	ı (4) Sequer	nce of Events
		14 Motor Ve	ubject shicle in Transport							
VEHICLE (Check if	Commercia	al)	Mah Liagana N				manarat			
Venicle   Motor Vehicle 1 1 Vehicle ii	n Transport	Hit and Run 1 No	veh License Number	State	Reg. Exp 23/Ma	orres Per 1r/2016	No			
Year Make Mi 2006 NISS A	Del Style	4D Color	AR Disabling	e  Est. Dam 800	age Towed	Due To Dam Yes	age Vehicle	Removed By TOWING (863	B)675	tation Rotation
Insurance Company					surance Policy I	Number				

Date of Crash 07/Nov/2015 04:35 PM	Date of Report 07/Nov/20	15 04:35 PM	Invest. Agency Report FHPF15	Number OFF069783	HSMV Cr	lumber 192700				
Name of Vehicle Owner (Check Box If Bu	siness)	Current Addre	ess (Number and Street)		City and	State		Zip Code		
Trailer License Number State	Reg. Expires F	Permanent Reg. VIN	N		Year N	Make	Length	Axles		
Trailer License Number State	Reg. Expires F	Permanent Reg. VIN	N		Year	Make	Length	Axles		
Vehicle Direction On Street, Ro Traveling: South	ad, Highway	SR	}-29		At Est. 5	Speed Post	ted Speed 60	Total Lanes 2		
CMV Configuration	Cargo	o Body Type		Area of	Initial Impact	Mos	st Damaged A	Area		
Comm GVWR/GCWR	Trailer Ty	vpe (trailer one)	Trailer Type (trailer two)	2 3 3 5 1 ((15 (( 18    1	6 7 18. Undercarrie 7 8 19. Overturn	ge 2 3 4		8. Undercarriage 9. Overturn		
Haz. Mat. Release Haz Mat. Placard	Number	Cla	SS	14 13 12 11	20. Windshiel 10 9 21. Trailer	d 14 13 12	2 11 10 0 2	20. Windshield 21. Trailer		
Motor Carrier Name	- 1	US DO	T Number		6					
Motor Carrier Addre	ess		City and	I State	N.	Zip Code	Phor	ne Number		
Comm/Non-Commercial Vehicle Body 1 1 Passe	Type Vo enger Car	ehicle Defects (one) <b>3 Tires</b>	Vehicle Defe	ects (two)	Emergency V 1 N	'ehicle Use <b>lo</b>	Speciual Fur 1 No Spec	nction of MV ial Function		
Vehicle Maneuver Action Trafficway 1 Straight Ahead 1 Two-Way	/, Not Divided	oadway Grade <b>1 Level</b>	Roadway Alignmen 1 Straight	t Most Harr 2 Colli	nful Event sion with Non-Fixe Object	ed Most H	larmful Event otor Vehicle	Detail in Transport		
Traffic Control Device For This Vehicle F 1 No Controls	irst (1) Sequence o 2 Collision with	of Events Seco Non-Fixed	ond (2) Sequence of Ever	nts Third (3) Se	equence of Events	Fourth (	4) Sequence	of Events		
	Objec 14 Motor Vehicle	τ in Transport		S.						
PERSON RECORD		-		0 10	Diath Cour	Dhama N				
1 1 1 Driver	1 venicie # iname			23/Ma	r/1989 2 Femal	e	umber	No No		
Address	City		State	0°	Zip C	ode				
Driver License Number State	e E	Expires 23/Mar/2019	DL Type 5 E/Operator	Req. End. 3 No Req Endorsement	Injury Severity 4 Incapac	citating	Ejection 1 Not I	Ejected		
Restraint System Air Bag Deplo 2 None Used -Motor 2 Not De Vehicle Occupant	yed Helm ployed	et Use	e Protection Seat 3 Not Applicable	ing Location Seat 1 Left	Seating Locati 1 Fro	on Row ont	Seating Loca 1 Not A	ation Other pplicable		
Drivers Actions at Time of Crash (first) 12 Drove too Fast for Cond	litions	Drivers Actions at T	Time of Crash (second)		Driver Distracted By 1 Not Distrac	v Vit	sion Obstruct 1 Vision Not	ion t Obscured		
Drivers Actions at Time of Crash (third)		Drivers Actions at T	Time of Crash (fourth)		Drivers Condition at 1	Time of Cras	sh Normal			
Suspected Alcohol Use Alcohol Tested 1 No 1 Test Not Given	d Alcohol Test T	ype Alcohol Tes	TResult BAC Susp	bected Drug Use 1 No	Drug Tested 1 Test Not Given	Drug Test T	Type Drug	j Test Result		
Source of Transport to Medical Facility 2 EMS	EMS Agency N HEN	Jame or ID DRY COUNTY EMS	EMS Run Nur	nber 023022	Medical Facilit	y Transporte EE MEMORIA	d To AL HOSPITA	L		
PERSON RECORD										
Person# Description 2 3 Passenger	Vehicle # Name 1			Date of E <b>31/Jar</b>	Birth Sex 1/1990 2 Femal	lnjury Se S Fatal	everity I (within 30 days)	Ejection 1 Not Ejected		
Address		City			I	State	Zip	Code		
Restraint System         Air Bag Deploy           3 Shoulder and Lap Belt         3 Deployed	yed Helm	et Use Ey	e Protection Seat 3 Not Applicable	ing Location Seat 3	Seating Locati 1	on Row	Seating Loca	ation Other 1		
Source of Transport to Medical Facility 1 Not Transported	EMS Agency N	Jame or ID	EMS Run Nur	nber	Medical Facilit	y Transporte	d To			
PERSON RECORD	<u>)</u>		I		I					
Person# Description 3 1 Driver	Vehicle # Name 2	)		Date of E 14/Jur	Birth Sex n/1957 1 Male	Phone N	lumber	Re-Exam <b>No</b>		
Address	City		State		Zip C	ode				
Driver License Number State		Expires 14/Jun/2021	DL Type 1 A	Req. End. <b>3 No Req</b> Endorsement	Injury Severity 3 Non-incap	acitating	Ejection 1 Not I	Ejected		
Restraint System <b>3 Shoulder and Lap Belt</b> <b>Used</b>	yed Helm ed-Front	et Use Ey	e Protection Seat	ing Location Seat <b>1 Left</b>	Seating Location Row         Seating Location Row           1 Left         1 Front         1 Not Ap			ation Other pplicable		
l Drivers Actions at Time of Crash (first) 1 No Contributing Actio	on I	Drivers Actions at T	l Time of Crash (second)		Driver Distracted By Vision Obstruction 1 Not Distracted 1 Vision Not Obscured					
Drivers Actions at Time of Crash (third)		Drivers Actions at T	Time of Crash (fourth)	1	Drivers Condition at Time of Crash 1 Apparently Normal					

Date of Crash 07/No	n ov/2015 04:3	35 PM	Date of Report 07/Nov/2015 0	94:35 PM	Invest. Agen	cy Report Num FHPF15OFF	ber 069783	HSMV Crash Report Number 85192700			
Suspected Alo 1 N	cohol Use Io	Alcohol Teste 1 Test No Given	ed Alcohol Test Type t	Alcohol Tes	t Result BA	C Suspecte	ed Drug Use No	Drug Teste 1 Test No	ed t Given	Drug Test Type	Drug Test Resul
Source of Tra	nsport to Me 2 EMS	dical Facility	EMS Agency Name HENDRY	e or ID COUNTY EMS	EM	S Run Number 0230	022	Medica	al Facility LEH	ransported To	HOSPITAL
WITNESSES	3		I					I			
Name			Address		City	/			s	State	Zip Code
VIOLATION	s										
Person# 1	Name			Florida Sta 316	tute Number . <b>183(1</b> )	Charge	TOO FAST	FOR CONI	DITIONS		Citation A4X1SRE
NARRATIVE				I	I					Q	
V01 was sout into the north rotated clock Vehicle Defec Name of Dece Date of Birth: Date of Death Death Pronou Traffic Homic Traffic Homic	thbound on hound lane wise and ca ets (V01) - Ti edent: 01/31/1990 1: 11/7/2015 h: 4:45 p.m. unced By: M ide Investig ide Case N	SR-29, just so The front of me to final res res - Both rea edic 3 - David ator: Corpora umber: FHP71	uth of SR-78. V02 was V02 struck the right si st blocking the northbo r Goodyear tires were Heflin Jr and Heidi Ar J J. Moore 5-60-005	northbound o de of V01. V01 ound lane, facir heavily worn. 7 nold	n SR-29, appro was redirected ng east. Fread depth of	oaching SR-78 d onto the eas	3. D01 lost co t grass shou t tire was 2/:	ontrol of VO ulder, where 32" or less.	1, and V e it came	01 rotated acro	oss the center line, acing west. V02
Photographs REPORTING	aken by: C	Corporal J. Mo	ore			<u>S</u>	S.				
ID/Badge # <b>3902</b>	Rank ar	nd Name	TPR M.K.	. REED	2		Department FLOR			Typ TROL	be of Department FHP
			otron Florida Citor	al tout Acting		in the second seco					



# FLORIDA TRAFFIC CRASH REPORT

T HIGHWAY SAFETY & MOTOR VEHICLES, TRAFFIC CRASH RECORDS

		ASH RECORDS		
NEIL KIRKMAN	BUILDING,	TALLAHASSEE,	FL	32399-0537

		(Elect	tronic Versior	ר)								
Date of Crash 28/Dec/2017 0	1:53 PM	ime of Cras 28/Dec/20	h 17 01:53 PM	Date of Report 08/Feb/2018 02:04	PM Invest.	Agency Repor FHPF1	t Number 70FF08767	78	HSMV Cr	ash Repor	t Number 85455740	
CRASH IDENT	IFIERS											
County Code 60	City Code 0	Cou	unty of Crash <b>(</b>	GLADES	Place or C	City of Crash UNINCORPO	RATED	Withir	n City Limits <b>No</b>	Time   28/  01	Reported Dec/2017 :56 PM	Time Dispatched 28/Dec/2017 02:12 PM
Time on Scene 28/Dec/2017 02:28 PM	Time Clear 28/Dec/2 P	red Scene 017 06:58 M	Completed <b>Yes</b>	Reason (if Investigatio	on NOT Com	pleted)				N	lotified By Law Ei	nforcement
ROADWAY INI	FORMATIC	ON		1								
Crash Occured C	On Street, Ro	oad, Highwa	ay SR-29			• At Street	Address#	0	At Lati 26.866	titude 52345582	and 84 -81.3	Longitude 66076469421401
At Feet	Or Miles 1.00	Dire D	ection North	OFrom Intersection V	Vith Street, R	oad, Highway CHAPPAR	AL AVENU	Е 5		S.	🗘 Or Fr	om Milepost #
Road System Ide	entifier 3 S	State		Type Of Shoulde	er 1 P	aved		Туре О	f Intersection 1	Not at Int	ersection	
CRASH INFOR	MATION (	Check if	Pictures Tal	ken) X					. nº v			
light Condition 1 Dayl	ight	Weathe	r Condition 1 Clear	Roadway Su	rface Conditi <b>1 Dry</b>	on School I	Bus Related	1 No	S M	anner Of C	Collision 3 Angl	e
First Harmful Eve	ent Type	Fi	rst Harmful Ev	vent 14	First Harmful <b>1</b>	Event Locatio On Roadway	n Y	Within Interch	lange Firs	t Harmful E	Event Relati 1 Non.Junc	on to Junction tion
Contributing Circ	umstances: 1 N	Road Ione		Contributing Circ	umstances:	Road	)	Contrib	uting Circums	stances: R	oad	
Contributing Circ	umstances: 1 N	Environme None	nt	Contributing Circ	umstances:	Environment		Contrib	uting Circums	stances: Ei	nvironment	
Work Zone Relat 1 No	ed Crash	In Work Zo	one	Туре С	Of Work Zone	,	30	Workers In	Work Zone	Law Er	nforcement	n Work Zone
VEHICLE (Che	ck if Com	mercial)	X				20					
Vehicle Motor V 2 1 Veh	ehicle Type	sport Hit	and Run 1 No	Veh License Number	Sta	ate Reg	g. Expires 1/Mav/2018	Permanen No	t Reg. VIN			
Year Make		Style	Color W	Extent of Dama	ge Est. D	Damage	owed Due T	o Damage es	Vehicle Rem	noved By		otation Rotation
Insurance Compa	any					Insurance Po	olicy Numbe	er				
Name of Vehicle	Owner (Che	eck Box If B	usiness)	Current A	ddress (Num	ber and Street	)		City and	State		Zip Code
Trailer License One:	Number	State	Reg. Expire	es Permanent Reg. Yes	VIN			Y	/ear   1968	Make FRUE	Length 20	Axles 2
Trailer License Two:	Number	State	Reg. Expire	es Permanent Reg.	VIN			Y	rear I	Make	Length	Axles
Vehicle Dire Traveling: <b>So</b>	ection C outh	n Street, R	oad, Highway	, R	SB-29				At Est.	Speed P	osted Spee	d Total Lanes
CMV Configuration	on 6			Cargo Body Type	- 0		Å	rea of Initial I	Impact		/lost Damag	ed Area
Comm GVWR/G	CWR	hs (11 793	ka) Tra	ailer Type (trailer one) Single Semi Trailer	Trailer Ty	/pe (trailer two)			<ul> <li>18. Undercarria</li> <li>19. Overturn</li> </ul>			<ul> <li>18. Undercarriage</li> <li>19. Overturn</li> </ul>
Haz. Mat. Releas	e Haz Ma	at. Placard	Number	Ś	Class		14 13		20. Windshiel 21. Trailer			20. Windshield 21. Trailer
Motor Carrier Na	me	1		US	DOT Numbe	r						
	Motor	Carrier Ado	ress			City ar	nd State			Zip Co	de f	hone Number
Comm/Mag Or	moreial 14	obials P'	Turk	Volisia Defente (	20)	Vahiel- P	foots (too)		Emorrar	(obiele L	Constitution of	Eurotice of MM
2	merciai V	20 Medium (more th (4,8	/ Type n/Heavy Truck an 10,000 lbs 536 kg))	(s 1 No	ne) ne	venicie De	ects (two)		Emergency V 1 I	No No	1 No S	pecial Function
Vehicle Maneuve 1 Straight A	er Action T head	rafficway 1 Two-Wa	ay, Not Divide	d Roadway Grade 1 Level	Roa	adway Alignme 1 Straight	ent Me t	ost Harmful E 2 Collision v Ol	vent with Non-Fix bject	ed Mos	t Harmful E <sup>.</sup> Motor Vehi	/ent Detail cle in Transport
Traffic Control De 1 No	evice For Th Controls	is Vehicle	First (1) Seque 2 Collision 14 Motor W	ence of Events n with Non-Fixed Object	l Second (2) Se	equence of Eve	ents Thir	d (3) Sequend	ce of Events	Fourt	h (4) Seque	nce of Events
	ok if Com	moreial										
	ehicle Type		and Run	Veh License Number	C+/	ate Dor	a Expires	Permanen	t Reg 17/IN			
1 1 Veh	icle in Tran	sport	1 No	von License Numper		2	ם. באטוופס 28/Feb/2018	No				

Date of	f Crash 28/Dec/2017 0	1:53 PM	D	ate of Rep 28/I	ort Dec/201	17 01:53 PN	A	Invest. /	Agency R Fl	eport Nui IPF170F	mber F087678		HSMV Ci	ash Rep	oort Nur 8545	nber 5740	
Insurar	nce Company								Insuran	ce Policy	Number						
Name of	of Vehicle Owne	r (Check	Box If Bus	iness)		Curre	ent Ad	dress (Numt	ber and S	treet)			City and	State			Zip Code
Trailor	Liconso Numb	or	State	Dog Evpir		ormanont C		ZINI					oar	Mako		path	Avloc
One:				neg. Expi	C3   1		icy.									ingun	
Trailer Two:	License Numb	ber	State	Reg. Expir	es P	ermanent H	leg.	VIN				Y	ear	Make	Le	ength	Axles
Vehicle Travelir	e Direction ng: North	On S	Street, Roa	id, Highway	,		9	SR-29					At Est.	Speed 10	Posted	I Speed 60	Total Lanes 2
CMV C	Configuration	•			Cargo	Body Type	!				Area o	of Initial I	npact		Most [	Damaged	Area
Comm	GVWR/GCWR			Tra	l ailer Tyj	pe (trailer o	ne)	Trailer Ty	pe (trailer	· two)	2 3 4 5 1 (15 ( 16	5 6 7 17 8	18. Undercarria 19. Overturn 20. Windshie	age 2 Id 1	3 (16	5 6 7 17 8	18. Undercarriage 19. Overturn 20. Windshield
Haz. M	at. Release H	az Mat.	Placard	Number			C	lass			14 13 12 1	1108	21. Trailer	ু হিন	13 12 1	1 10 0	21. Trailer
Motor (	Carrier Name						US D	OT Number	•					5			
	۸	Antor Ca	rrier Addre	<u>ee</u>					C	ity and St	late		<u></u>	Zin (	Code	Pho	ne Number
			iner / talare.	55									120V	29,		THE	ne Namber
Comm/	/Non-Commercia	al  Vehi	icle Body T 2 Passer	ype nger Van	Ve	hicle Defec	ts (on I <mark>Non</mark> e	e) e	Vehicl	e Defects	s (two)		Thergency \	/ehicle L <b>No</b>	Jse Sp	beciual Fu 1 No Spe	nction of MV cial Function
Vehicle 10	e Maneuver Actic Making U-Turn	on Traft <b>1</b>	ficway <b>Two-Way</b> ,	Not Divide	ed Ro	adway Gra 1 Le	de vel	Roa	dway Alig 1 Stra	nment aight	Most Ha	armful Ev Ilision v Ob	/ent /ith Non-Fix ject	ed 1	ost Hari 14 Moto	mful Even or Vehicle	t Detail in Transport
Traffic	Control Device F	or This V	Vehicle Fir	rst (1) Sequ	ience of	f Events	Se	cond (2) Se	quence o	f Events	Third (3)	Sequenc	e of Events	Fo	urth (4)	Sequence	e of Events
			1	14 Motor V	Object ehicle i	in Transpo	rt		~		0.	Ó					
PERSO					1						S		-				
Person 1	# Description	Driver	V	/ehicle # <b>1</b>	Name					00	Date o 28/F	f Birth eb/1936	Sex 1 Male	Pho	one Nun	nber	Re-Exam Yes
Addres	s			City				0	State	2		-	Zip (	Code			-1
Driver I	License Number		State		E	xpires 28/Feb/2	2019	DL Type 6 E/C	e Dper - Re	st Red	g End 3 No Req Endorseme	nt Ir	jury Severity 4 Incapa	/ citating	Ej	ection 1 Not	Ejected
Restrai 3 Shou	int System ulder and Lap B	elt Air E	Bag Deploy 2 Not Dep	red bloyed	Helme	et Use		Eye Protecti 3 Not App	on Nicable	Seating	Location Sea 1 Left	at S	eating Locat 1 Fr	ion Row ont	Se	eating Loc	ation Other
Drivers	Actions at Time 3 Failed	of Crast to Yield	n (first) Right.of.V	Vay		Drivers Act	ions a	t Time of Cr	ash (secc	ond)		Driver 1	Distracted B	/ cted	Visio 1 V	n Obstruc Vision No	tion <b>t Obscured</b>
Drivers	Actions at Time	of Crast	h (third)			Drivers Act	ions a	t Time of Cr	ash (four	th)		Drivers	Condition a 6 Sei	t Time o zure, Ep	f Crash <b>bilepsy</b> ,	Blackou	t
Suspec	cted Alcohol Use 1 No	Alco 3 1	hol Tested Fest Given	Alcohol	Test Ty Blood	rpe Alco	ohol T 2 Coi	est Result npleted	BAC 0.000	Suspect	ted Drug Use 1 No	Drug 3 T	Fested est Given	Drug T	fest Typ Blood	e Dru	g Test Result <b>2 Negative</b>
Source	of Transport to 2 EN	Medical I IS	Facility	EMS Aç	jency N GLAI	ame or ID DES COUN		us	EMS RL	in Numbe	er	N	ledical Facili L	ty Trans EE MEM	ported 1	To HOSPITA	AL
PERSO	ON RECORD					<u>D</u>											
Person 2	# Description 3 Pas	senger	V	ehicle # 1	Name	9					Date o 22/J	f Birth I <b>ul/1945</b>	Sex 2 Fema	le 5	ry Seve Fatal (v day	rity vithin 30 /s)	Ejection 1 Not Ejected
Addres	s		ľ		10.0	Ô	City							Stat	te	Zip	Code
Restrai 3 Shou	int System ulder and Lap B Used	Air E	Bag Deploy 2 Not Dep	red bloyed	Helme	et Use		Eye Protection 3 Not App	on blicable	Seating	Location Sea 3	at S	eating Locat <b>1</b>	ion Row	Se	eating Loc	ation Other
Source	of Transport to I 1 Not Tran	Medical sported	Facility	EMS Ag	gency N	ame or ID			EMS RU	in Numbe	er	M	ledical Facili	ty Trans	ported 7	Го	
PERSO	ON RECORD		S.								<b>.</b>	I					
Person 3	# Description 1 E	Driver 🤇		/ehicle # 2	Name						Date o 07/D	f Birth ec/ <b>1965</b>	Sex 1 Male	Pho	one Nur	nber	Re-Exam <b>No</b>
Addres	s			City					State				Zip (	Code			
Driver I	License Number		State		E	xpires 07/Dec/3	2019	DL Type		Red	q. End. 1 Yes	Ir	jury Severity 1 No	/ ne	Ej	ection 1 Not	Eiected
Restrai 3 Shou	nt System ulder and Lap B Used	Air E	ag Deploy <b>1 Not App</b>	ed <b>licable</b>	Helme	et Use		Eye Protection 3 Not App	on olicable	Seating	Location Sea	at S	eating Locat 1 Fr	ion Row ont	Se	eating Loc	ation Other
Drivers	Actions at Time 1 No C	of Crasi ontribu	h (first) ting Actio	n	1	Drivers Act	ions a	t Time of Cr	ash (seco	ond)		Driver 1	Distracted By	ted	Visio 1	n Obstruc Vision No	tion t Obscured

Date of Crash 28/Dec/	2017 01:53 PM	Dat	te of Report 28/Dec/20	3 PM	Invest. Ag	jency Re FH	port Number PF17OFF087678	HSMV Crash Report Number 85455740				·	
Drivers Actions a	at Time of Crash (th	nird)		Drivers	Actions at Ti	me of Cras	sh (fourth	)	Drivers (	Condition at	t Time of Cras Apparently I	sh Norma	
Suspected Alcoh 1 No	nol Use Alcohol 3 Test	Tested Given	Alcohol Test T 1 Blood	ype J	Alcohol Test 2 Compl	Result E leted	BAG 0.000	Suspected Drug Use 1 No	Drug Te 3 Te:	sted st Given	Drug Test T 1 Bloo	ype d	Drug Test Result 2 Negative
Source of Transp 1 No	port to Medical Faci ot Transported	ility	EMS Agency N	Name or	ID	E	EMS Ru	Number	Me	dical Facilit	ty Transported	d To	
VIOLATIONS					T								
Person# Ni 1	ame				Florida Statu 316.	ute Numbe .1515	r Char	U-TURN - IMPRO	PER/UNS	AFE/PRO	HIBITED	Cita	ation A8UAGCE
NARRATIVE													
4106 TRO Vehicle 1 (V01) improper "U-tur came to final res Name of Deceas Date of Birth: 07 Date of Death: 1 Time of Death: 1 Tronounced By THI Case: FHP7 THI Investigator Photographs Ta	Was traveling nort n" in front of V02. st on south grass sed: 1995 2/28/2017 22:7 P.M. : Paramedic Dillioi 17-60-010 : Corporal James ken By: Corporal	ANT h on SR- As a res shoulder m McMah A. Harris James A	29, north of Ch 29, north of Ch ult, the front por r, facing south non of Hendry C 4. Harris	napparal ortion o with its	ĞHWAY PAT I Avenue. Ve f V02 struck front agains EMS	hicle 2 (VC the right s the right s	9-344-1; 92) was side of \ side of	30 Jan 01, 2018 raveling south on \$ 01. V01 came to fin V01.	6R-29, noi al rest on	rth of Chap the south	oparal Avenu grass should	ie. V01 der, fa	made an cing west. V02
								6	<u></u>				
ID/Badge #	Bank and Name							Department	0			Type o	f Department
3389			CORPOR	AL J.A.	HARRIS			FLO	RIDA HIG	HWAY PA	TROL		FHP
			K C C C C C C C C C C C C C C C C C C C	Voreix Sol	South Contraction of the second secon	S TO NO							



# FLORIDA TRAFFIC CRASH REPORT

HIGHWAY SAFETY & MOTOR VEHICLES,
TRAFFIC CRASH RECORDS
NEIL KIRKMAN BUILDING, TALLAHASSEE, FL 323

		(Elec	tronic Versio	n)	X			NEIL KIF	RKMAN	BUILDING	G, TALL	AHASS	EE, FL	32399	0537	
Date of Crash 22/Nov/2017 0	5:35 AM	Time of Cras 22/Nov/20	sh 17 05:35 AM	Date o 08/0	of Report Oct/2019 09:27 AN	Invest. /	lgency Re FH	eport Numb PF170FF0	oer 078745		HSMV (	Crash Re	sh Report Number 85605345			
CRASH IDENT	IFIERS	1									1					
County Code 60	City Coc	le Co O	unty of Crash	GLADE	S	Place or G	ity of Cras UNINCO	sh RPORATE	D	Within C	City Limits No	Tin 2	ne Repor 22/Nov/20 05:47 Al	ted T 017 M	ime Dispatched 22/Nov/2017 05:53 AM	
Time on Scene 22/Nov/2017 06:26 AM	Time Clo 22/Nov	eared Scene //2017 12:20 PM	Completed <b>Yes</b>	Reaso	on (if Investigation I	NOT Comp	leted)						Notified L	l By .aw Enf	orcement	
ROADWAY IN	FORMA	TION	-	1									2			
Crash Occured (	On Street,	Road, Highw	ay SR-29				O At Sti	reet Addres	ss#	0,	O At La 26.892	attitude 2 <b>3501111</b>	56698	and L - <b>81.34</b> :	ongitude 3810027465196	
At Feet	Or Miles	i Dir .00	ection North	OFro	om Intersection With	n Street, Ro	ad, Highv CHA	way I <b>PARRAL</b> /	AVE	5		.0	0	Or Fror	n Milepost #	
Road System Ide	entifier	2 State		<u>'</u>	Type Of Shoulder	1 D	wood			Type Of Ir	ntersectio	n 1 Not of	Interese	tion		
		V (Check if	Pictures Ta	ken)		164	iveu				- dr	TNOTAL	Intersec			
light Condition		Weathe	r Condition	,	Roadway Surfa	ce Conditio	n Sch	ool Bus Re	lated			Manner C	)f Collisio	n		
5 Dark-Not	t Lighted	4 F	og, Smog, Sr	noke	1[	Dry			1 No	6	5		2 Fro	nt to Fi	ont	
First Harmful Ev	ent Type	F	irst Harmful E	vent 14	Fir	st Harmful 1 (	Event Loc On Roady	ation	With	in Interchan No	ige Fir	st Harmf	ul Event I 1 Non	Relatior	to Junction	
Contributing Circ	umstance	s: Road 1 None			Contributing Circun	nstances: F	Road			Contributi	ng Circun	nstances	: Road			
Contributing Circ	umstance 2 Weath	s: Environme	ent ns		Contributing Circun	nstances: E	nvironme	ent		Contributi	ng Circun	nstances	: Environ	ment		
Work Zone Rela 1 No	ted Cra	sh In Work Z	one	I	Type Of	Work Zone		(	<u>so</u> w	l /orkers In W	ork Zone	Law	Enforce	ment In	Work Zone	
	ck if Co	mmercial)	X			6		<u>Š</u>		$\bigcirc$						
Vehicle Motor V 2 1 Veh	/ehicle Ty i <b>icle in Tr</b> a	pe Hit ansport	and Run 1 No	Veh L	icen <u>se Number</u>	Sta	te FL	Reg. Expi 31/Dec.	res   F /2017	Permanent F No	Reg. VIN					
Year Make	Model	Style	Color		Extent of Damage	Est. D	amage	Towed [	Due To Da	amage V	ehicle Re	moved B	ly Olympia	Rota	ation	
Insurance Comp	. 470 any			7HI	Disability		Insuranc	e Policy N	umber		CLEW		OWING			
Name of <u>Vehicle</u>	Owner (C	heck Box If E	Business)		Current Add	ress (Numt	per and St	ireet)			City an	d State			Zip Code	
Trailer License One:	e Number	State	Reg. Expir	es P	Permanent Reg. V	INS				Yea	ar	Make	Lei	ngth	Axles	
Trailer License Two:	e Number	State	Reg. Expir	es P	Permanent Reg. V	IN				Yea	ar	Make	Lei	ngth	Axles	
Vehicle Dir Traveling: No	ection orth	On Street, F	Road, Highway	5	<b>F</b> s	R-29					At Est	. Speed 60	Posted	Speed 60	Total Lanes 2	
CMV Configurati	on	2		Cargo	Body Type	3			Area	of Initial Imp	pact		Most D	amageo	l Area	
Comm GVWR/G 2 10.001-2	CWR 6.000 lbs	(4.536-11.79	Tri 13 kg)	ailer Ty	rpe (trailer one)	Trailer Ty	pe (trailer	two)			18. Undercar 19. Overturi		[15][10]		18. Undercarriage 19. Overturn	
Haz. Mat. Relea	se Haz	Mat. Placard	Number	Ġ	Cla	ass			14 13 12	11 10 8 2	20. Windshi 21. Trailer		13 12 11		20. Windshield 21. Trailer	
1 Motor Carrier Na	ime	1		8		DT Number										
							0									
	Mot	or Carrier Ade	dress				Ci	tv and Stat	e			Zip	Code	Ph	one Number	
Comm/Non-Com 2	mercial	Vehicle Bod 20 Medium (more th	y Type n/Heavy Truc ian 10,000 lbs	ks Ve	ehicle Defects (one 1 None	)	Vehicle	e Defects (	two)	En	nergency 1	Vehicle   No	Use Sp 1	eciual F No Sp	unction of MV ecial Function	
Vehicle Maneuvo 1 Straight A	er Action	Trafficway	ay, Not Divide	ed Ro	oadway Grade <b>1 Level</b>	Roa	dway Alig <b>1 Str</b> a	nment iight	Most H	larmful Eve 1 Non-Ce	nt ollision	N	lost Harn 2	nful Eve Fire/Ex	nt Detail plosion	
Traffic Control D	evice For Control	This Vehicle s	First (1) Sequ 2 Collisio	ience o	f Events Sec Non-Fixed	ond (2) Se 2 Fire	quence of /Explosie	f Events on	Third (3)	) Sequence	of Events	i Fo	ourth (4) s	Sequen	ce of Events	
			14 Motor V	ehicle	ו in Transport											
VEHICLE (Che	ck if Co	mmercial)			I				•							
Vehicle Motor V 1 1 Veh	/ehicle Ty i <b>icle in Tr</b> a	pe Hit ansport	and Run 1 No	Veh L	icense Number	Sta	te	Reg. Expi 07/Sep	res F / <b>2018</b>	Permanent F <b>No</b>	Reg. VIN					
Year Make 2007 FORI	Model DEXPED	Style	T Color	LK	Extent of Damage Disabling	Est. D	amage 3000	Towed [	Due To Da Yes	amage V	ehicle Re	moved B MS TOW	iy ING	Rota	ation Rotation	
Insurance Comp	any						Insuranc	e Policy N	umber							

Date of Ci 2	ash 2/Nov/2017 05:3	5 AM	Date of Repo 22/N	rt ov/2017 05:35	АМ	Invest. A	lgency Re FHF	port Num PF17OFF	ber 078745		HSMV Cr	ash Repo	rt Number 85605345		
Name of V	/ehicle Owner (C	heck Box If Bu	siness)	Cı	irrent Addr	ess (Numb	ber and Str	eet)			Citv and	State			Zip Code
Trailer L One:	icense Number	State	Reg. Expire	s Permaner	it Reg. VII	N				Yea	r	Make	Length		Axles
Trailer L Two:	icense Number	State	Reg. Expire	s Permaner	nt Reg. VII	N				Yea	r I	Make	Length		Axles
Vehicle Traveling:	Direction South	On Street, Roa	ad, Highway		SF	<b>}-29</b>					At Est.	Speed F 50	Posted Spe 60	ed	Total Lanes <b>2</b>
CMV Con	liguration	•		Cargo Body Ty	/pe				Area of	Initial Imp	act		Most Dama	aged A	rea
Comm G\	WR/GCWR		Tra	iler Type (traile	r one)	Trailer Typ	oe (trailer t	wo)	2 3 4 5		8. Undercarria 9. Overturn	*ge 2 3	4 5 8	7 18 6 18	3. Undercarriage 9. Overturn
Haz. Mat.	Release Haz	Mat. Placard	Number		Cla	ss			14 13 12 11		0. Windshie 1. Trailer		12 11 10	B 2	0. Windshield 1. Trailer
Motor Car	rier Name					TNumber				0		.SV			
	Moto	or Carrier Addre	ss				Cit	/ and Sta	te	2_		Zip Co	ode	Phon	e Number
Comm/No	n-Commercial	Vehicle Body	Type Itility Vehicle	Vehicle De	fects (one)		Vehicle	Defects	(two)	Err	iergency \	/ehicle Us	e Speciu	al Fun	ction of MV
Vehicle M	aneuver Action	Trafficway	Not Divide	Roadway (	Grade	Road	dway Align	ment	Most Har	mful Ever	jt Iliaian	Mos	st Harmful	Event	Detail
Traffic Co	ntrol Device For	This Vehicle F	rst (1) Seque	ence of Events	Seco	ond (2) Sec	quence of	Events	Third (3) S	equence	of Events	Four	th (4) Seq	uence (	of Events
	T NO CONTOIS	5	2 Comstor ( 14 Motor Ve	Dbject Dicle in Trans	ea port	2 Fire	Explosio		S	0					
PERSON	RECORD								<u> </u>						
Person# [ 1	escription <b>1 Driv</b>	er	/ehicle # <b>1</b>	Name					Date of 12/De	Birth <b>c/1994</b>	Sex 1 Male	Phon	e Number		Re-Exam <b>No</b>
Address			City			0	State	2		D	Zip C	Code			
Driver Lice	ense Number	State		Expires 12/De	ec/2020	DL Type 5 E/	Operator	Req	. End. 3 No Req Endorsemen	lnju 5 F	ry Severity atal (with	′ in 30 day	s) Ejection 1	on Not E	jected
Restraint 3 3 Should	System er and Lap Belt Used	Air Bag Deploy 88 Deplo Unkn	/ed yment own	Helmet Use	Ey	e Protectio 3 Not App	on licable	Seating (	ocation Seat 1 Left	Sea	ting Locat 1 Fre	ion Row ont	Seatin	g Loca	tion Other
Drivers Ac	tions at Time of 25 Failed to K	<u>l</u> Crash (first) <b>(eep in Proper</b>	Lane	Drivers	Actions at <sup>-</sup>	Time of Cra	ash (secor	id)		Driver Dis 8	stracted By 8 Unknow	/ /n	Vision Ot	structio 8 Fo	on g
Drivers Ac	tions at Time of	Crash (third)		Drivers	Actions at	Time of Cra	ash (fourth			Drivers C	ondition a	t Time of ( 88 Un	) Crash <b>known</b>		-
Suspected	l Alcohol Use U <b>nknown</b>	Alcohol Tested 3 Test Giver	Alcohol 1	est Type / Blood	Alcohol Tes 2 Com	t Result pleted	BAC 0.000	Suspecte 88 U	ed Drug Use nknown	Drug Te: 3 Tes	sted t Given	Drug Te	st Type Blood	Drug 1	Test Result Positive
Source of	Transport to Mee 1 Not Transpo	l dical Facility orted	EMS Age	ency Name or I	D		EMS Rur	ı Number		Mec	lical Facili	ty Transpo	orted To		
PERSON	RECORD			-	5		1								
Person# [ 2	escription <b>1 Driv</b>	er	/ehicle # 2	Name					Date of 25/Ja	Birth <b>n/1986</b>	Sex 1 Male	Phon P	e Number		Re-Exam <b>No</b>
Address			City				State				Zip C	Code			
Driver Lice	ense Number	State		Expires 25/Ja	n/2019	DL Type 5 E/	Operator	Req	. End. 3 No Req Endorsemen	lnju 5 F t	ry Severity atal (with	′ in 30 day	s) Ejection 1	on Not E	jected
Restraint 2 None Vehic	System Used -Motor e Occupant	Air Bag Deploy 1 Not App	/ed blicable	Helmet Use	Ey	e Protectio 3 Not App	on licable	Seating I	_ocation Seat 1 Left	Sea	ting Locat 1 Fre	ion Row ont	Seatin	g Loca	tion Other
Drivers Ac	tions at Time of <b>1 No Con</b>	l Crash (first) t <b>ributing Actio</b>	ň	Drivers	Actions at <sup>-</sup>	Time of Cra	ash (secor	ıd)		Driver Dis 1 N	stracted By ot Distrac	/ ted	Vision Ot	structio 8 Fo	on g
Drivers Ac	tions at Time of	Crash (third)		Drivers	Actions at 7	Time of Cra	ash (fourth	)		Drivers C	ondition a <b>1</b>	t Time of ( Apparen	Srash itly Norma	I	
Suspected	l Alcohol Use <b>1 No</b>	Alcohol Tester 3 Test Giver	l Alcohol T	est Type # Blood	Alcohol Tes 2 Comp	t Result pleted	BAC 0.000	Suspecte 1	ed Drug Use No	Drug Te: 3 Tes	sted t Given	Drug Te	st Type Blood	Drug 1	Test Result Positive
Source of	Transport to Mee 1 Not Transpo	dical Facility orted	EMS Age	ency Name or I	D		EMS Rur	1 Number		Mec	lical Facili	ty Transpo	orted To	1	
PERSON	RECORD						•								
Person# [ 3	escription <b>3 Passe</b>	nger	/ehicle # 2	Name					Date of <b>09/Ja</b>	Birth n/ <b>1988</b>	Sex 2 Fema	le Injury 4 li	Severity ncapacitat	ing	Ejection 1 Not Ejected
Address					City						1	State		Zip (	Code

Date of Crash 22/Nov/2017 05:35 AM	Dat	te of Repo 22/N	rt ov/2017 05:35 AM		Invest. Age	ncy R FH	eport Number IPF170FF078	3745		HSMV Crash	ו Report I 8	Number 5605345		
Restraint System Air B 2 None Used -Motor Vehicle Occupant	ag Deployed 1 Not Applic	t cable	Helmet Use	Eye 3	Protection Not Applica	able	Seating Loca	ation Seat 3	Sea	ting Location 1	Row	Seating L	ocatio	on Other
Source of Transport to Medical F 2 EMS	acility	EMS Ag	ency Name or ID GLADES COUNTY	/ EMS	EN	AS Ru G	in Number CSO17CAD	028238	Mec	lical Facility T FLORIDA	ransport A HOSPI	ed To TAL LAKE	PLA	CID
PERSON RECORD														
Person# Description 5 3 Passenger	Vel	nicle # 2	Name					Date of Birth 23/May/19	55	Sex 2 Female	Injury S 4 Inc	everity apacitating	g E	jection 1 Not Ejected
Address			C	Sity						I	State		Zip Ci	ode
Restraint System Air B 2 None Used -Motor Vehicle Occupant	ag Deployed 1 Not Applic	i cable	Helmet Use	Eye <b>3</b>	Protection Not Applica	able	Seating Loca	ation Seat 77	Sea	ting Location 88	Row	Seating L	.ocatio 3	on Other
Source of Transport to Medical F 2 EMS	acility	EMS Ag	ency Name or ID HENDRY COUNT	YEMS	EN	/IS Ru	in Number EMS17CAD0	04715	Mec	lical Facility T	ransport IENDRY	ed To REGIONA	L	
PERSON RECORD										D.				
Person# Description 4 3 Passenger	Vel	nicle # 2	Name					Date of Birth 30/Mar/194	47	Sex 1 Male	Injury S 5 Fata	everity al (within 3 days)	<b>10</b>	jection 1 Not Eiected
Address			C	Sity					2	<u>, v</u>	State	<b>,</b>	Zip Ci	ode
Restraint System Air B 1 Not Applicable (non-motorist)	ag Deployed 1 Not Applic	t cable	Helmet Use	Eye 3	e Protection Not Applica	able	Seating Loca	ation Seat	Sea	ting Location 77	Row	Seating L	.ocatio 3	on Other
Source of Transport to Medical F 1 Not Transported	acility	EMS Age	ency Name or ID		EN	MS RL	in Number		Mec	lical Facility T	ransport	ed To		
PERSON RECORD								8	7					
Person# Description 6 3 Passenger	Vel	nicle # 2	Name				× 00	Date of Birth 27/Jul/197	6	Sex 1 Male	Injury S 4 Inc	everity apacitating	a E	jection 1 Not Ejected
Address			C	)ity	0						State		Zip Ci	ode
Restraint System Air B 1 Not Applicable (non-motorist)	ag Deployed 1 Not Applic	t cable	Helmet Use	Eye 3	Protection Not Applica	able	Seating Loca	ation Seat 77	Sea	ting Location 77	Row	Seating L	ocatio 3	on Other
Source of Transport to Medical F 2 EMS	<sup>=</sup> acility	EMS Ag	ency Name or ID HENDRY COUNTY	EMS	EN	AS RU	in Number EMS17CAD0	04715	Мес	lical Facility T LEE	ransport MEMOR	ed To IAL HOSP	ITAL	
NON VEHICLE PROPERTY	DAMAGE				6,									
Vehicle# Person# Property Dam	age - Other ROADWAY	Than Vehi	cle Est. Amount B	Susines Yes	s Owner's M	lame DOT	A	ddress 2981 PINE IS	SLAN	City ID RD	« State CAPE C	ORAL FL	Z	ip Code <b>33903</b>
NAKHATIVE	A Street of Stre	THIO OCT	A Sidrah Four											

Vehicle# Person# Property Damage - Other Than Vehi	e Est. Amount Business	Owner's Name	Address	City & State	Zip Code
ROADWAY	3000 Yes	FDOT	2981 PINE ISLAND RD	CAPE CORAL FL	33903

#### NARRATIVE



ID/Badge # Department Type of Department FHP 1744 FLORIDA HIGHWAY PATROL



# FLORIDA TRAFFIC CRASH REPORT

HIGHWAY SAFETY & MOTOR VEHICLES,
TRAFFIC CRASH RECORDS
NEIL KIRKMAN BUILDING, TALLAHASSEE, FL 323

	LUM	NG FORIN	U 5 (I	Electronic	Version	) )	JATE			٦	NEIL KI	RKMAN	BUILDIN	G, TAL	LAHAS	SEE, FL 3	32399-	0537
Date of 0 03/Jul	Crash / <b>2018 03</b> :	11 PM	Time of 03/Ju	Crash <b>I/2018 03</b> :	11 PM	Date o 28/N	of Report Iov/2018 09:32 Al	M Inv	vest. Ageno	cy Rep FHP	ort Num F180FF	ber <b>044188</b>		HSMV	' Crash R	leport Numb 871776	oer 808	
CRASH	IDENTI	FIERS																
County ( 6	Code i <b>0</b>	City Cod 4	e 10	County o	f Crash G	LADE	s	Place	or City of MC	Crash DORE	HAVEN		Within	City Limi <b>No</b>	ts T	ime Report 03/Jul/20 03:16 PM	ed Ti 18	me Dispatched 03/Jul/2018 03:22 PM
Time on 03/Ju 04:13	Scene I/2018 7 PM	Time Cle 03/Jul	eared Sce /2018 07 PM	ene Com :46	pleted Yes	Reaso	on (if Investigation	NOT	Completed	)					I	Notified La	By Iw Enfo	orcement
ROADW		ORMAT																
Crash O	cured Or	n Street,	Road, Hi	ghway SR	-29				0 /	At Stre	et Addre	ss#	0	O At 26.8	Lattitude 6440999	e 9999999	ind La - <b>81.367</b>	ongitude 930000000001
At Feet		Or Miles .t	50	Direction Sou	th	<b>O</b> From	m Intersection Wit	th Stre	et, Road, H CHA	lighwa PPAR	ay RAL AVE	NUE NW	5		.0	0	Or From	n Milepost #
Road Sy	stem Ider	ntifier	Stata			Г	Type Of Shoulder			4			Type Of I	Intersect	ion 1 Not :	at Intercent	ion	
CRASH	INFORI		(Chec	k if Pictu	ires Tak	(en)				<u> </u>				- 0'	TNOL	at intersect		
light Con	dition 1 Daylig	yht	We	ather Conc 3	dition Rain	,	Roadway Surfa	ace Co Wet	ondition	Scho	ol Bus R	elated 1 No		<u>,                                    </u>	Manner	Of Collision 2 Fror	t to Fr	ont
First Har	mful Ever	nt Type	I	First Ha	armful Ev	ent 14	Fi	irst Ha	rmful Even 1 On R	t Loca oadwa	ition ay	With	in Intercha No	nge F	irst Harn	nful Event F <b>1 Non</b> .	elation	to Junction
Contribu	ting Circu	mstance:	s: Road I None	_	-	0	Contributing Circu	mstand	ces: Road		0		Contribut	ting Circu	umstance	es: Road	_	
Contribu	ting Circu	mstance: 2 Weath	s: Enviro er Cond	nment itions			Contributing Circu	mstand	ces: Enviro	onmen	t	.0.	Contribut	ting Circu	umstance	es: Environn	nent	
Work Zo	ne Relate 1 No	d Cras	sh In Wo	rk Zone			Type Of	Work	Zone		X	So N	/orkers In V	Vork Zor	ie La	w Enforcen	nent In '	Work Zone
VEHICL	E (Cheo	k if Co	mmerci	al) 📃					<b>O</b>		_20							
Vehicle 2	Motor Ve 1 Vehic	hicle Typ cle in Tra	be ansport	Hit and R	un lo	Veh Li	icense Number 9353IK		State FL	F	Reg. Exp 31/Dec	ires    :/ <b>2019</b>	Permanent No	Reg. V	IN	2FTJW35M	1NCA:	5556
Year 1992	Make FORD	Model F35 PICKU	0 JP	тк	Color BL	U.	Extent of Damage Disabling	E	st. Damaç <b>1000</b>	je D	Towed	Due To D Yes	amage \	/ehicle F B	Removed	By WING	Rota	tion Rotation
Insuranc	e Compa	ny					. 01		Inst	irance	Policy N	lumber						
Name of	Vehicle (	)wner (C	heck Box	<u>( If B</u> usine:	ss)		Current Add	dress (	Number ar	nd Stre	eet)			City a	and State	:		Zip Code
Trailer One:	License I	Number	Sta	te Re	g. Expire	s Pi	ermanent Reg.						Ye	ar	Make	Len	gth	Axles
Trailer Two:	License I	Number	Sta	te Re	g. Expire	s P	ermanent Reg.	AN					Ye	ar	Маке	Len	gth	Axles
Vehicle Traveling	g: Direction	ction <b>th</b>	On Stre	et, Road, I	lighway	)		SR-29	Ο				I	At E	st. Speec 60	I Posted S	Speed 60	Total Lanes 2
CMV Co	nfiguratio	n				Cargo	Body Type					Area	of Initial Im	npact		Most Da	amaged	Area
Comm G	iVWR/GC	WR		~	Trai	iler Typ	pe (trailer one)	Trail	er Type (tr	ailer tv	wo)	2 3 4	5 6 7 17 8	18. Underd 19. Overte 20. Winds	arriage um shield		8 7	18. Undercarriage 19. Overturn 20. Windshield
Haz. Mat	t. Release	e Hazl	Mat. Plac	ard N	umber	s S	C	lass				14 13 12	11 10 9	21. Tralle		14 13 12 11	10 9	21. Trailer
Motor Ga	arrier Nam	ie			420	Š		OT Nu	Imber									
		Moto	or Carrier	Address	<u>e</u>					City	and Sta	te			Zi	p Code	Ph	one Number
Comm/N	on-Comn	nercial	Vehicle	Body Type 3 Picku	p	Ve	hicle Defects (one 1 None	e) 9	Ve	ehicle	Defects	(two)	E	mergeno	y Vehicle 1 No	e Use Spe	ciual Fi No Spe	unction of MV cial Function
1 St	raight Ah	Action ead	1 Tafficwa 1 Two	ay o-Way, No	t Divideo	d Ro	adway Grade 1 Level		Roadway	Alighi Straig	ment ght	Most I 2 C	Harmful Eve Collision wi Obj	ent ith Non- ect	Fixed	Most Harm 14 Motor	tul Evei Vehicl	e in Transport
Traffic C	ontrol Dev <b>1 No</b>	vice For Controls	This Vehi s	icle First ( 2	1) Seque Collision	ence of with I Object	f Events Se Non-Fixed	cond (	2) Sequen	ce of E	Events	Third (3)	) Sequence	e of Even	its	Fourth (4) S	equenc	e of Events
				14	Notor Ve	hicle i	n Transport											
VEHICL	E (Cheo	k if Co	mmerci	al) 🗌					1-		_		_					
Vehicle 1	Motor Ve 1 Vehic	hicle Typ cle in Tra	be ansport	Hit and R	un No	Veh Li	cense Number		State	■  F	Reg. Exp 21/Oc	ires     /2018	Permanent No	Reg. V				
Year         Make         Model         Style         Color         Extent of Damage         Est. Damage         Towed Due To Damage         Vehicle Remove           2013         BMW         X5         UT         DBL         Disabling         Ist. Damage         Towed Due To Damage         Vehicle Remove         BILLS Towed Due Towe						Removed	By WING	Rota	Rotation									

Insurance Company

Insurance Policy Number

Date of	Crash 03/Jul/	2018 03:1 <sup>.</sup>	1 PM	[	Date of Re 0	eport 3/Jul/2	2018 03:11 PM	И	Invest. A	gency Ro FH	eport Num IPF180FF	iber <b>-044188</b>		HSMV (	Crash I	Report 8	Number 37177808		
Name o	fVehicle	Owner (C	heck	Box If Bus	siness)		Curn	ent Ac	ddress (Numb	er and S	treet)			Citv an	d Stat	e			Zip Code
Trailer One:	License	e Number	!	State	Reg. Ex	pires	Permanent I	Reg.	VIN				Y	ear	Make	;	Length		Axles
Trailer Two:	License	e Number	:	State	Reg. Ex	pires	Permanent I	Reg.	VIN				Y	ear	Make	9	Length		Axles
Vehicle Travelin	g: S	ection outh	On S	street, Roa	ad, Highw	ay	1		SR-29				I	At Est	. Spee 60	ed Po	sted Spec	ed	Total Lanes <b>2</b>
CMV Co	onfigurat	on				Ca	rgo Body Type	е				Area	of Initial Ir	npact		М	ost Dama	ged A	rea
Comm (	GVWR/G	GWR				Trailer	Type (trailer o	one)	Trailer Typ	be (trailer	two)	2 3 4 1 (15 (( 18	5 8 7 17 B	18. Undercar 19. Overtun	niege .	2 🗿	4 5 8 18 17	7 11 6 1	8. Undercarriage 9. Overturn
Haz. Ma	t. Relea	se Haz	Mat. F	Placard	Numbe	r		6	Class			14 13 12	11 10 9	20. windsh 21. Trailer		14/13	12 11 10	0 2	0. windshield 1. Trailer
Motor C	arrier Na	ime						US	DOT Number				6		5				
		Moto	or Car	rier Addre	ess					Ci	ty and Sta	ite	S	7	$\frac{1}{z}$	Zip Cod	le	Phon	e Number
Comm/t	lon-Con	mercial	Vehi 16	cle Body T ( <b>Sport</b> ) U	Fype Itility Veh	icle	Vehicle Defe	cts (or 1 Nor	ne) 1e	Vehicl	e Defects	(two)	E	Emergency 1	Vehic <b>No</b>	le Use	Speciua 1 No	al Fun Speci	ction of MV al Function
Vehicle 16 Lea	Maneuv ving Tra	er Action ffic Lane	Traffi 1	icway Two-Way	, Not Divi	ded	Roadway Gra 1 Le	ade evel	Road	dway Alig <b>1 Str</b> a	nment aight	Most F 2 C	larmful Ev	ent /ith Non-Fi	xed	Most 14 I	Harmful E Motor Vel	vent vicle i	Detail n Transport
Traffic C	Control D <b>1 N</b>	evice For o Controls	 This V S	/ehicle Fi	irst (1) Se 45 (	quence Cross (	e of Events Centerline	s	econd (2) Se 14 Motor Ve	quence o hicle in 1	f Events Fransport	Third (3) 8 R	) Sequenc an into W	e of Events ater/ Cana	i I	Fourth	ı (4) Sequ	ence	of Events
PERSC	N REC	ORD											2						
Person#	Descrip	otion 1 Driv	er	\	/ehicle # <b>1</b>	Na	me				×	Date 04/	of Birth Mar/1978	Sex 2 Fem	ale	Phone	Number		Re-Exam <b>No</b>
Address					City				0	State		6	D	Zip	Code				
Driver L	icense N	lumber		State	FL		Expires 04/Mar/	2026	DL Type 5 E/	Operator	Req	. End. 3 No Re Endorsem	q 5 ent	jury Severi Fatal (wit	ty hin 30	days)	Ejectio 1	n Not E	jected
Restrain 3 Shou	it Syster Ider and Used	ו Lap Belt	Air B 3	ag Deploy B Deploye	/ed d-Front	Hel	I Imet Use	C	Eye Protection 3 Not App	on licable	Seating	ocation Se 1 Left	eat S	eating Loca 1 F	ation F <b>ront</b>	low	Seating	Loca	tion Other
Drivers	Actions a	at Time of Failed to K	Crash Crash	n (first) in Proper	Lane		Drivers Ac	tions	at Time of Cra	ash (seco	nd)	-	Driver I	Distracted I 88 Unkno	By wn	١	√ision Obs 2 Incle	structi ement	on Weather
Drivers	Actions	at Time of	Crash	ı (third)			Drivers Ac	tions	at Time of Cra	ash (fouri	h)		Drivers	Condition	at Tim 8	e of Cr 8 Unki	ash nown		
Suspect	ed Alcol 1 No	iol Use	Alcol 3 T	hol Testec est Giver	l Alcoh	ol Test 1 Blo	Type Alc	ohol 7 2 Co	Test Result Impleted	BAC 0.000	Suspecte	ed Drug Us I No	e Drug T 3 Te	ested est Given	Dru	ig Test 1 Blo	Type ood	Drug 1	Test Result Positive
Source	of Trans 1 N	port to Mee ot Transpo	dical F orted	acility	EMS	Agency	Name or ID	R.	0	EMS Ru	n Numbei	r	M	edical Faci	lity Tra	anspor	ted To		
PERSC	N REC	ORD						5											
Person#	Descrip	otion 1 Driv	er	\ \	/ehicle # 2	Na	me					Date 15//	of Birth <b>Aug/1985</b>	Sex 1 Ma	le	Phone	Number		Re-Exam <b>No</b>
Address					City					State				Zip	Code				
Driver L	icense N	lumber		State	FL	.8	Expires 15/Aug/	2022	DL Type	1 A	Req	. End. 3 No Ree Endorsem	q ent	jury Severi 4 Incap	ty acitati	ing	Ejectio 1	n Not E	jected
Restrair 2 Noi Veh	it Syster ne Used icle Occ	-Motor upant	Air B	ag Deploy 1 Not App	/ed blicable	Hel	Imet Use		Eye Protectio 3 Not App	on licable	Seating I	Location Se 1 Left	eat S	eating Loca 1 F	ation F ront	low	Seating	Loca	tion Other
Drivers	Actions	at Time of ( <b>1 No Con</b>	l Crash tribut	i (first) ting Actio	n K		Drivers Ac	tions	l at Time of Cra	ash (seco	nd)		Driver I	Distracted I Not Distra	By acted	\	vision Obs 2 Incle	structi ement	on Weather
Drivers	Actions	at Time of (	Crash	(third)	<u>.</u>		Drivers Ac	tions	at Time of Cra	ash (fourt	h)		Drivers	Condition	at Tim 1 App	e of Cr parenti	ash y Normal		
Suspect	ed Alcol 1 No	iol Use	Alcol	hol Testec Test Not Given	l Alcoh	ol Test	Type Alc	ohol 7	Fest Result	BAC	Suspecte	ed Drug Us I No	e Drug 1 1 Tes	Tested t Not Give	n Dru	ıg Test	Туре	Drug	Test Result
Source	of Trans	port to Mea 2 EMS	l dical F	acility	EMS	Agency GL	y Name or ID ADES COUO	ΝΤΥ Ε	EMS	EMS Ru	n Number CSO18C	, AD018100	       M	edical Faci	lity Tra LEE N	anspor IEMOF	ted To RIAL HOS		L
PERSC		ORD								I									
Person# 4	Descri	otion 3 Passei	nger		/ehicle # 2	Na	me					Date 29/	of Birth May/2007	Sex 1 Ma	le	injury S 5 Fat	Beverity al (within days)	30	Ejection 1 Not Ejected
Address	i			1				City								State		Zip	Code

Date of Crash 03/Jul/2018 03:11 PM	A Dat	e of Repo 03/J	rt ul/2018 03:11 PM		Invest. Agency Report Number FHPF180FF044188				HSMV Crash Report Number 87177808			
Restraint System Air 3 Shoulder and Lap Belt Used	Bag Deployed 1 Not Applic	able	Helmet Use	Eye 3 I	Protection Not Applicable	Seating Loc	ation Seat 3	Seating Location 2	Row	Seating Loca	ation Other	
Source of Transport to Medical 2 EMS	Facility	EMS Age	ency Name or ID GLADES COUNTY	'EMS	EMS Ru	n Number CSO18CAD	018100	Medical Facility	ransported	I To L HOSPITA	L	
PERSON RECORD												
Person# Description 3 3 Passenger	r r	nicle # 2	Name				Date of Birth 16/Jun/198	Sex 2 Female	Injury Sev 4 Incap	verity pacitating	Ejection 1 Not Ejected	
Address			С	ity					State	Zip	Code	
Restraint System Air 5 Lap Belt Only Used	Bag Deployed 1 Not Applic	able	Helmet Use	Eye 3	Protection Not Applicable	Seating Loc	ation Seat 2	Seating Location 1	Row	Seating Loca	ation Other	
Source of Transport to Medical 2 EMS	Facility	EMS Age	ency Name or ID GLADES COUNTY	'EMS	EMS Ru G	n Number CSO18CAD	018100	Medical Facility	ransported MEMORIA	I To L HOSPITA	L	
PERSON RECORD					I			0	2			
Person# Description 5 3 Passenger	r Veh	nicle # 2	Name				Date of Birth 28/Oct/200	5 Sex 1 Male	Injury Sev 4 Incap	verity pacitating	Ejection 1 Not Ejected	
Address			С	ity				01	State	Zip	Code	
Restraint System Air	Bag Deployed		Helmet Use	Eye	Protection	Seating Loc	ation Seat	Seating Location	Row S	Seating Loca	ation Other	
3 Shoulder and Lap Belt Used	1 Not Applic	able		3	Not Applicable		3 6	<b>1</b>		U		
Source of Transport to Medical 2 EMS	Facility	EMS Age	ency Name or ID GLADES COUNTY	'EMS	EMS Ru	n Number CSO18CAD	018100	Medical Facility 1	Transported	I To L HOSPITA	L	
NARRATIVE							<u>v</u>					
ID Number Rank Name 3992 TROOPER M. D. : Vehicle 01 (V01) was southbo Vehicle 02 (V02) was northbo V01 crossed over the center I As a result, the front of V02 c V01 came to final rest, facing V02 came to final rest, facing V02 came to final rest, facing Name of Deceased: Date of Birth: 03/04/1978, Date of Beath: 3:23 pm. Pronounced By: Paramedic J THI Case: FHP718-60-003. THI Investigator: Corporal Ju Photographs Taken By: Corp Name of Deceased: Date of Birth: 5/29/2007 Date of Death: 1:37 pm. Pronounced By: Doctor Drew THI Case: FHP718-60-003.	SILL F ound on State und on State ine and rotate ollided with th east, in a war northeast, or ohn Riggs (G stin Close ID oral Gabriel C	pp / Post FLt Road 29, Road 29, ed counte he right s ter filled on the east lades Co ≇ 835. Cornier IC	Officer Agency DRIDA HIGHWAY P , approximately 0.5 approximately 0.5 erclockwise, on the ide of V01. drainage ditch, on t a grass shoulder of unty EMS).	Pho PATROI miles : miles : wet ro he eas: State F	one Number Da 239-344-1730 south of Chappa south of Chappar adway, into the r t shoulder of Sta Road 29.	tte Created Jul 07, 20 ral Avenue N al Avenue N northbound te Road 29.	18 IW. Jane, directly in	n front of VO2.				
Photographs Taken By: Corp Name of Deceased: Date of Birth: Stillborn Date of Death: 07/08/2018. Time of Death: 07/08/2018. Time of Death: 2:32 am. Pronounced By: Doctor Lee THI Case: FHP718-60-003. THI Investigator: Corporal Ju Photographs Taken By: Corp	stin Close ID oral Gabriel C	¢ 835. Cornier ID	# 1148									

ID/Badge # <b>2569</b>	Rank and Name	CORPORAL J.D. CLOSE	Department FLORIDA HIGHWAY PATROL	Type of Department FHP



### FLORIDA TRAFFIC CRASH REPORT

# HIGHWAY SAFETY & MOTOR VEHICLES, TRAFFIC CRASH RECORDS NEIL KIRKMAN BUILDING, TALLAHASSEE, FL 32399-0537

		(Electronic	Version)					<b>,</b>		· , ·	
Date of Crash     Time of Crash     Date of Report     Invest. Agency F       15/Jul/2021 02:13 AM     15/Jul/2021 02:13 AM     14/Feb/2022 03:05 PM     F							nber F034123	HSM	V Crash Re	eport Number 88371042	
CRASH IDENTIFIER	RS										
County Code City 60	Code 0	County of	Crash GL	ADES	Place or City L	y of Crash ININCORPORAT	ED	Within City Lin No	nits Tin	ne Reported 15/Jul/2021 02:29 AM	Time Dispatched 15/Jul/2021 02:31 AM
Time on Scene Time 15/Jul/2021 15 03:13 AM	Cleared /Jul/2021 PM	Scene Comr 01:38 Y	leted R 'es	eason (if Investigation	NOT Comple	eted)		1	•	Notified By Law E	nforcement
ROADWAY INFOR	<b>JATION</b>	1	•							2	
Crash Occured On Stro	eet, Road	, Highway STATE R	OAD 29			At Street Addreet	ess#		: Lattitude 856493163:	and 931699 -81.3	Longitude 74623071853904
At Feet Or N 1000	liles	Direction Sout	h C	From Intersection Wi	th Street, Roa C	id, Highway XHAPPARAL AVI		9	.0	O Or Fr	om Milepost #
Road System Identifier	3 Stat	e		Type Of Shoulder	1 Pav	red		Type Of Intersed	ction 1 Not at	Intersection	
CRASH INFORMAT	ION (Ch	eck if Pictu	res Take	en) X				0	· · · · · · · · · · · · · · · · · · ·		
light Condition 5 Dark-Not Light	ed	Weather Cond <b>1 C</b>	ition Clear	Roadway Surf	ace Condition Dry	School Bus F	Related 1 No	1301	Manner C	Df Collision 2 Front to	Front
First Harmful Event Ty	pe	First Ha	mful Ever	nt F 14	irst Harmful E 1 O	vent Location n Roadway	Within	Interchange No	First Harmf	ful Event Relati 1 Non.June	on to Junction c <b>tion</b>
Contributing Circumsta	nces: Roa <b>1 Non</b>	ad Ie		Contributing Circu	imstances: Ro	ad	1	Contributing Cire	cumstances	: Road	
Contributing Circumsta	nces: Env 1 Non	vironment Ie		Contributing Circu	imstances: En	vironment		Contributing Cire	cumstances	: Environment	
Work Zone Related <b>1 No</b>	Crash In '	Work Zone		Type O	Work Zone		Wor	rkers In Work Zo	ine Law	v Enforcement	In Work Zone
VEHICLE (Check if	Comme	ercial) X		·	0	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~			•		
Vehicle Motor Vehicle 2 1 Vehicle in	Type Transpo	Hit and Ru	in V o	eh License Number	State	FL Reg. Exp FL 29/Ju	pires Pe II/2021	rmanent Reg. No	/IN		
Year Make Mo	del S	Style TK	Color WHI	Extent of Damag Disabling	e Est. Dai	mage Towed	Due To Dan Yes	nage Vehicle	Removed E	By R	otation Rotation
Insurance Company						Insurance Policy	Number				
Name of Vehicle Owne	r (Check	Box If Busines	s) X	Current Ad	dress (Numbe	er and Street)		Citv	and State		Zip Code
Trailer License Num One:	ber i	State Reg	J. Expires	Permanent Reg. Yes				Year 2005	Make TRI	M Length	Axles 2
Trailer License Numl Two:	per :	State Reg	ı. Expires	Permanent Reg.	VIN			Year	Make	Length	Axles
Vehicle Direction Traveling: North	On S	treet, Road, H	ighway	STAT	E ROAD 29			At I	Est. Speed 60	Posted Spee 60	d Total Lanes 2
CMV Configuration	7		c	argo Body Type	3		Area of	f Initial Impact		Most Damag	jed Area
Comm GVWR/GCWR 3 More than 26	,000 lbs (	(11,793 kg)	Traile Ta	er Type (trailer one) Indem Semi Trailer	Trailer Type	e (trailer two)		17 θ 18. Unde 17 θ 19. Over 20. Win	turn tahield		18. Undercarriage 19. Overturn 20. Windshield
Haz. Mat. Release H 1	laz Mat. F	Placard Nu 1	mber		lass		14 13 12 11	1 10 9 21. Trail	er 14	13 12 11 10	21. Trailer
Motor Carrier Name				USE	OT Number						
	Notor Car	rier Address		2		City and Sta	ate		Zip	Code	Phone Number
Comm/Non-Commerci 2	al Vehic 20 (r	cle Body Type Medium/Heav more than 10, (4,536 kg	y Trucks 000 lbs ))	Vehicle Defects (on 1 Non-	e) E	Vehicle Defects	(two)	Emerger	icy Vehicle 1 No	Use Speciua 1 No S	I Function of MV Special Function
Vehicle Maneuver Acti 1 Straight Ahead	on Traffi 1	icway Two-Way, Not	Divided	Roadway Grade <b>1 Level</b>	Road	way Alignment 1 Straight	Most Ha 2 Col	rmful Event Ilision with Nor Object	-Fixed	Most Harmful E 14 Motor Veh	vent Detail icle in Transport
Traffic Control Device 1 No Con	For This V trols	/ehicle First ( 2 C 14 M	l) Sequen Collision v Ot lotor Vehi	ce of Events Se with Non-Fixed bject icle in Transport	econd (2) Seq 27 Gua	uence of Events rdrail Face	Third (3) S	Sequence of Eve	nts Fo	ourth (4) Seque	ence of Events
VEHICLE (Check if	Comme	ercial)									

ľ	Vehicle	Motor Veh	icle Type		Hit and Ru	n Veh	License Number	State	Reg. Expires	Permanent Re	g. VIN	
	1	i venici	e m trans	ροπ	1 No	D	Y969KL	FL	18/Nov/2021	NU	1B3ES56C0	4D634472
Γ	Year	Make	Model	Style		Color	Extent of Damage	Est. Damage	Towed Due To	Damage Vel	nicle Removed By	Rotation
	2004	DODG	NEON		4D	RED	Disabling	7000	Yes		BILL'S TOWING OF LAB	Rotation

Insurance Company     Insurance Policy Number       Name of Vehicle Owner (Check Box If Business)     Current Address (Number and Street)       Trailer     License Number       State     Reg. Expires       Permanent Reg.     VIN	
Name of Vehicle Owner (Check Box If Business)     Current Address (Number and Street)     City and State       Trailer     License Number     State     Reg. Expires     Permanent Reg.     VIN     Year     Make     Length	
Trailer License Number State Reg. Expires Permanent Reg. VIN Year Make Length	Zip Code
	Axles
Trailer License Number State Reg. Expires Permanent Reg. VIN Year Make Length	Axles
Vehicle         Direction         On Street, Road, Highway         At Est. Speed         Posted Speed         60	ed Total Lanes 2
CMV Configuration Cargo Body Type Area of Initial Impact Most Dama	ged Area
Comm GVWR/GCWR     Trailer Type (trailer one)     Trailer Type (trailer two)     2 3 4 5 6 7 18. Undercarriage 19. Overturn 19. State 10 17 18. Undercarriage 19. Overturn 19. Overturn 19. State 10 17 18. Undercarriage 19. State 10 18 18 18 18 18 18 18 18 18 18 18 18 18	7 18. Undercarriage 8 19. Overturn 20. Windshield
Haz. Mat. Release Haz Mat. Placard Number Class 14 13 12 11 10 3 21. Trailer 14 13 12 11 10	9 21. Trailer
Motor Carrier Name US DOT Number	
Motor Carrier Address City and State Zip Code	Phone Number
Comm/Non-Commercial         Vehicle Body Type         Vehicle Defects (one)         Vehicle Defects (two)         Emergency Vehicle Use         Special           1 Passenger Car         1 None         1 None         1 No         1 No	al Function of MV Special Function
Vehicle Maneuver Action         Trafficway         Roadway Grade         Roadway Alignment         Most Harmful Event         Most Harmful 14 Motor Ve           16 Leaving Traffic Lane         1 Two-Way, Not Divided         1 Level         1 Straight         2 Collision with Non-Fixed         14 Motor Ve	Event Detail hicle in Transport
Traffic Control Device For This Vehicle         First (1) Sequence of Events         Second (2) Sequence of Events         Third (3) Sequence of Events         Fourth (4) Sequence of Events           1 No Controls         45 Cross Centerline         14 Motor Vehicle in Transport         30 Concrete Traffic Barrier         Fourth (4) Sequence of Events	ence of Events
PERSON RECORD	
Person#     Description     Vehicle #     Name       1     1     Date of Birth     Sex       28/Jun/1966     2 Female	Re-Exam <b>No</b>
Address City State Zip Code	
Driver License Number     State     Expires     DL Type     Req. End.     Injury Severity     Ejection       29/Jul/2022     5 E/Operator     5 E/Operator     3 No Req Endorsement     5 Fatal (within 30 days)     1	n Not Ejected
Restraint System         Air Bag Deployed         Helmet Use         Eye Protection         Seating Location Seat         Seating Location Row         Seating Locating Location Row         Seating Location Row <td>Location Other</td>	Location Other
Drivers Actions at Time of Crash (first)         Drivers Actions at Time of Crash (second)         Driver Distracted By         Vision Ob           21 Wrong Side of Wrong Way         Drivers Actions at Time of Crash (second)         Driver Distracted By         Vision Ob	struction n Not Obscured
Drivers Actions at Time of Crash (third) Drivers Actions at Time of Crash (fourth) Drivers Condition at Time of Crash 88 Unknown	
Suspected Alcohol Use 2 YesAlcohol Tested 3 Test GivenAlcohol Test Type 77 Other, Explain in NarrativeAlcohol Test Result 2 CompletedBAC 0.140Suspected Drug Use 2 YesDrug Tested 3 Test GivenDrug Test Type 77 Other, Explain in Narrative	Drug Test Result 1 Positive
Source of Transport to Medical Facility EMS Agency Name or ID EMS Run Number Medical Facility Transported To 1 Not Transported	
PERSON RECORD	
Person#     Description     Vehicle #     Name       2     1 Driver     2     1 Driver     1 Male	Re-Exam No
Address City State Zip Code	
Driver License Number State Expires DL Type Req. End. Injury Severity Ejection 12/Mar/2028 1 A 1 Yes 4 Incapacitating 1	n Not Ejected
Restraint System       Air Bag Deployed       Helmet Use       Eye Protection       Seating Location Seat       Seating Location Row       Seating Location Row         3 Shoulder and Lap Belt       2 Not Deployed       Helmet Use       Seating Location       Seating Location Seat       Seating Location Row       Seating Location Ro	Location Other
Drivers Actions at Time of Crash (first)     Drivers Actions at Time of Crash (second)     Driver Distracted By     Vision Ob       1 No Contributing Action     Driver Second     1 Not Distracted     1 Vision Ob	struction n Not Obscured
Drivers Actions at Time of Crash (third) Drivers Actions at Time of Crash (fourth) Drivers Condition at Time of Crash 1 Apparently Norma	1
Suspected Alcohol Use     Alcohol Tested     Alcohol Test Type     Alcohol Test Result     BAC     Suspected Drug Use     Drug Tested     Drug Test Type       1 No     1 Test Not Given     Alcohol Test Result     BAC     Suspected Drug Use     1 No     1 Test Not Given     Drug Test Type	Drug Test Result
Source of Transport to Medical Facility 2 EMS Agency Name or ID GLADES COUNTY EMS COUNTY EMS AGENCY CO	OSPITAL
NON VEHICLE PROPERTY DAMAGE	
Vehicle#         Person#         Property Damage - Other Than Vehicle         Est. Amount 10000         Business Yes         Owner's Name FDOT         Address         City & State           Vehicle#         FDOT         5893 ENTERPRISE PKWY         FORT MYERS F	Zip Code L 33905

Date of Crash 15/Jul/2	2021 02:13 AM	Date of Report 15/Jul/2021 02:13 AM	Invest. Agency Report Number FHPF210FF034123	HSMV Crash Report Number 88371042
Vehicle# Person#	Property Damage - ROA	Other Than Vehicle Est. Amount Busines DWAY 1000 Yes	SS Owner's Name Address FDOT 5893 ENTERPRIS	City & State Zip Code E PKWY FORT MYERS FL 33905
NARRATIVE				
NARRATIVE ID Number Ra 4587 TPR Vehicle 01 (V01) Chapparal Aven counterclockwis the west should Name of deceas: Date of birth: 06, Date of birth: 06, Date of beath: 0 Time of Death: 2 Pronounced by: THI Case Number THI Investigator Photographs tak ID Number Ra 3992 CPL This report was	ank Name S. REILLY was traveling souti ue NW. V01 veered se onto the west she er of State Road 29 ed Driver: 7/15/2021 2:26 AM Paramedics Tony I ar: FHP721-60-001 : Corporal Matthew (en by: Corporal Matthew (en by: Corporal Matthew M. D. SILL updated to show th	Troop / Post Officer Agency PI F FLORIDA HIGHWAY PATROL hoound on State Road 29, south of Chap into V02's lane of travel, causing the fror oulder of State Road 29. V02 jackknifed a facing north. V02 came to final rest on th Bevis and David Heflin Sr. of Glades Cour Sill tithew Sill of the Florida Highway Patrol Troop / Post Officer Agency PI F FLORIDA HIGHWAY PATROL e toxicology results for D01.	hone Number Date Created 239-344-1730 Jul 15, 2021 paral Avenue NW. Vehicle 02 (V02) was travelin it of V01 to collide with the front of V02. V01 was ind collided with a guardrail on the east should be east shoulder of State Road 29 facing west.	g northbound on State Road 29, approaching s redirected upon collision and began to rotate ar of State Road 29. V01 came to final rest on
REPORTING C	DFFICER			
ID/Badge # <b>3992</b>	Rank and Name	CPL M. D. SILL	Department	Type of Department HPF FHP
	Retific	to sole the sole to the sole t		

Date of Grash 15/Jul/2021 02:13 AM	Date of Report 15/Jul/2021 02:13 AM	FHPF21OFF034123	HSMV Crash Report Number 88371042
	State Roa	rd 29	
L chap	saral Avenue NW		
V01 traveling :		V02 cols	ding with guardrait, V02 final rest
V01 final	rest		
			22 4.05
Concrete barra	· - / / / /	Concrete barrier	Solv Solv
Paved Shoulder	- <b>-</b> / <i>}/</i> -/-	——— Paved Shouider	
		/01 colliding with V02	
j			
1			
	H		
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-4			
	AL ROL 2		

# APPENDIX G FEMA Floodplain Maps





#### 81°22'29.24"W 26°48'31.87"N

NUMBER

120095

PANEL

0460

## **FLOOD HAZARD INFORMATION**

### SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT



## **NOTES TO USERS**

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at https://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates, refer to the Flood Insurance Study Report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Basemap information shown on this FIRM was provided in digital format by the United States Geological Survey (USGS). The basemap shown is the USGS National Map: Orthoimagery. Last refreshed October, 2020.

This map was exported from FEMA's National Flood Hazard Layer (NFHL) on 8/5/2021 12:32 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at https://www.fema.gov/media-library/assets/documents/118418

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

### SCALE

Map Projection: GCS, Geodetic Reference System 1980; Vertical Datum: No elevation features on this FIRM For information about the specific vertical datum for elevation features, datum conversions, or vertical monuments used to create this map, please see the Flood Insurance Study (FIS) Report for your community at https://msc.fema.gov





MAP NUMBER 12043C0460C EFFECTIVE DATE September 26, 2014

# APPENDIX H Historical Drainage Reports

# **DRAINAGE COMPLAINT**

## **S.R. 29**

# From North of S.R. 78 to Chaparral Slough (M.P. 4.709 – 6.877)

# Reference No. 05090-1

Prepared for:



Florida Department of Transportation District 1

Need rearpus Field rearpus with Labelle people

Prepared By:



AIM Engineering & Surveying, Inc. 5802 Breckenridge Pkwy, Suite 100 Tampa, FL 33610

January 2010

### **TABLE OF CONTENTS**

1.0	INTRODUCTION	1
2.0	BACKGROUND RESEARCH	2
3.0	FINDINGS	3
4.0	HYDRAULIC ANALYSIS	4
5.0	RECOMMENDATIONS	5

### LIST OF APPENDICES

Appendix A	Original Drainage Complaint
Appendix B	FDOT Straight Line Diagram
Appendix C	FEMA Maps
Appendix D	Correspondence
Appendix E	Photos
Appendix F	Drainage Maps
Appendix G	Design Calculations
Appendix H	Analysis of Flooding Report
Appendix I	Cost Estimate

### **1.0 INTRODUCTION**

The Florida Department of Transportation (FDOT), District One, has gathered drainage complaints from residents and maintenance staff throughout the district and compiled them into a drainage complaint inventory. The recorded complaints were ranked in importance based on frequency and severity of flooding as well as roadway classification and traffic data. This document specifically focuses on one particular drainage complaint in Glades County, Florida along State Road (S.R.) 29 from north of S.R. 78 to Chaparral Slough (see **Appendix A**). A location map is contained in **Figure 1**.

S.R. 29 is a major north/south connector between Labelle and Harrisburg. If this roadway is forced to close, traffic will have to be detoured either west to S.R. 731 or east along S.R. 78 to S.R. 25. Either option will require traffic to detour miles out of the way to other facilities that may also encounter flooding during these times.

The purpose of this report is to document the causes for the flooding problems along S.R. 29 as reported in the drainage complaint. It will also provide potential solutions, cost estimates and recommendations for mitigation.



### Figure 1: Location Map

### 2.0 BACKGROUND RESEARCH

S.R. 29 is a two-lane undivided roadway with 5-ft paved shoulders and roadside ditches. The traffic volumes are medium. Water periodically encroaches the roadway as close as six feet from the travel lane between Lone Pine Creek and Cypress Branch bridges (Milepost (M.P.) 5 and M.P. 6). During severe storm events, water has overtopped the roadway.

Adjacent properties and ditches continually flood. The surrounding terrain is relatively flat. While the flow patterns are draining south to the Caloosahatchee River, there are no well defined channels. The adjacent properties are owned by Lykes Brothers and little to no maintenance has been performed over the years.

There are two cross drains and two bridges within the study area. As illustrated in the Straight Line Diagrams (**Appendix B**), the bridge over Lone Pine Creek is at M.P. 4.709, a triple 30" RCP at M.P. 5.500, a triple 36" RCP at M.P. 6.237 and the bridge over Cypress Branch is at M.P. 6.848. In 1977, the cross drains were extended out 24' on both sides to the right of way as part of a resurfacing project. The existing pipes remained in place.

These crossings are located within FEMA Floodplain Zone A (see **Appendix C**). Flooding of S.R. 29 has been observed at this location as far back as 1970. The drainage inventory provided by the FDOT notes that severe flooding and roadway encroachment has only occurred twice in the past 10 years during the rainy season. During their investigation, FDOT Drainage collected rainfall data from August  $18^{th}$  through August  $21^{st}$ , year unknown, and conducted a field review. The gage data obtained is shown in **Table 1** below.

Duration	Rainfall	Frequency (per FDOT Drainage)
1 Day	3.0 in.	10 Year
2 Day	6.4 in.	25 Year
4 Day	7.7 in.	10 Year

### Table 1: Rainfall Data

The FDOT staff noted overtopping in the ditches and that the surrounding properties were flooded. The bridges at Lone Pine Creek and Cypress Branch were free of debris and appeared to be flowing full. The triple 30" cross drain and the triple 36" cross drain appeared to be free of debris as well. No damage was observed at the drainage structures.

A scour report was obtained for bridge number 050033 (Cypress Branch Bridge). Cypress Branch is a riverine waterway that flows perennially. The velocity measured at the bridge during a field review was less than one foot per second. All piles and bents have no known lengths or embedments. The measured scour over 10 years is 5.8 feet. Therefore, the bridge was given a medium priority scour susceptible rating. There were no scour reports available for the Lone Pine Creek Bridge number 050035.

A field meeting was held with FDOT Maintenance staff of July 1, 2009 (see **Appendix D**). Mr. John Anderson pointed out the areas of concern and how the historical and existing flow patterns function. He indicated that flooding in the area is a major concern and occurs on a yearly basis,

more often than originally indicated. In fact, the road flooded over the July  $4^{th}$  weekend and the traffic had to be rerouted. Another field review was conducted on July 22, 2009 to follow up on the recent flooding and to photograph the area (see **Appendix E**).

### 3.0 FINDINGS

An initial overview of the drainage complaint revealed that the rainfall frequencies measured in the field by FDOT drainage do not closely match the frequencies provided in the Drainage Manual for the area. See a comparison in **Table 2** below.

Duration	Frequency	Rainfall (per FDOT review)	Rainfall (Precipitation Data- Drainage Manual)
1 Day	10 Year	3.0 in.	7.0 in.
2 Day	25 Year	6.4 in.	9.5 in.
4 Day	10 Year	7.7 in.	9.0 in.

Table 2:	Rainfall	Comparison
----------	----------	------------

According to FDOT Maintenance, the major flooding occurs at the two cross drains. The triple 30" cross drain floods more severely than the triple 36" cross drain. The two bridges at Lone Pine Creek and Cypress Branch appear to be functioning properly. The field reviews and analysis confirm this initial observation.

The original 1956 drawings, more recent resurfacing plans, and Lone Pine Creek Bridge widening plan sets were obtained from the FDOT Maintenance Office. The original drainage maps were reviewed and still appear to be appropriate (see **Appendix F**).

Based on the performed cross drain analysis (see Appendix G), the triple 30" cross drain appears to be undersized. This cross drain backs up and in turn floods the triple 36" cross drain upstream. Both cross drains are severely silted in, as well as the adjacent ditches, the flow path along the right of way fence. The photographs taken on July 22, 2009 (see Appendix E) show the water elevations approaching the top of the endwalls. The amount of silt could not be measured due to the amount of standing water present. The build up of sediment could easily be blocking flow through the pipe and even preventing water from reaching the outfall. The joints where the pipes were extended could have shifted or collected debris and in turn also cause the cross drains to flood. Furthermore, the high water elevation for the triple 30" pipes is at 35.3 ft. This is almost two feet higher than the high water elevation for the triple 36" and both bridges. The high water values were taken from the original 1956 drainage map (Appendix F), and still seem reasonable based on present conditions. Flow lines for both cross drains are at an approximate elevation of 30 feet. Pictures taken in the field show the water level to be at the crown of the pipe for both cross drains. This would equate to a water elevation of approximately 33 feet. Since the pictures were taken during the wet season, the water elevation closely coincides with the high water elevations.

The side drain pipe to the north of the Lone Pine Creek Bridge may also be undersized. According to the photographs taken on July 22, 2009, the cross drain is holding back a

significant amount of water. The Analysis of Flooding Problem for S.R. 29 West of Chaparral Slough, PBS&J, October 1999 (Appendix H), identifies this side drain pipe at 30" and documents flooding problems here as well. The report suggests double 72" pipes be installed to address the flooding. Currently, this side drain consists of two 54" CMP pipes based on correspondence from Labelle Maintenance (Appendix D). This can be verified from the pictures located in Appendix E. There is a significant amount of runoff coming to this side drain from the surrounding area. During the field review, the side drain pipes were flooded more significantly on the northern side than the southern side. Even though the existing conditions suggest a blockage, the side drain was under water and it could not be determined if there was anything blocking the pipes. However, based on the findings of the crossdrains in the area, it is assumed that the side drain is full of silt and needs to be cleaned as well.

### 4.0 HYDRAULIC ANALYSIS

A drainage map was developed based on the original plans. Due to the flat and unchanged topography of the area, these historic drainage maps did not appear to have any significant changes to them and therefore were closely modeled for this drainage analysis. This map is located in **Appendix F** as well as an updated drainage map that breaks apart the areas specifically traveling to the cross drains and side drain. All elevations and areas were taken from State Job No. 0504-201, Fiscal Year 1956 and verified from FPID 193957-2-52-01, Fiscal Year 2004. Using the rational method, flow rates for 25 year, 50 year, and 100 year were determined. The 500 year design flow was then approximated using a log graph of the flow rate vs. the design frequency. These values were then entered into the HY-8 program to analyze the existing cross drains. All calculations can be found in **Appendix G**. **Table 3** summarizes the results.

Cross Drain	Overtopping Flow Rate	Frequency
Triple 30 inch	30.42 cfs	<< 25 Year
Triple 36 inch	193.86 cfs	> 500 Year

Table 3: Existing Overtopping Rates

The existing triple 30" cross drain is clearly undersized. It overtops the roadway well before the 25 year design flood of 94.35 cfs. The existing triple 36" pipe has enough capacity to handle flow rates higher than the 50 year design flood.

Several scenarios were analyzed for improving the cross drain including upsizing each existing culvert separately and both of them together. The downstream bridges and culverts on S.R. 78 were also reviewed to determine if these locations were restricting the flow and causing flooding along S.R. 29. It appears that the least impact to the function of traffic along S.R. 29 would be to replace the existing triple 30" pipes with quadruple 48" pipes. For this improvement, the overtopping flow rate for this cross drain would be 113.3 cfs. This is between the 50 year and 100 year design floods. Also with the existing roadway elevation at elevation 35.45, the proposed 48" pipes should be set at a lower flow line than the existing triple 30" pipes resulted in overtopping before the 25 year flood frequency as did triple 48" pipes. This analysis is based upon the structures acting independently. However, based on the flat terrain observed during

field reviews and review of old plans, during larger rain events the structures may function together. Increasing the pipe sizes could also cause problems further down stream. Water that is currently being restricted will have free range flow that could impact the drainage structures along S.R. 78.

The existing double 54" side drain north of Lone Pine Creek was also analyzed. This pipe was originally a single 30" pipe according to the *Analysis of Flooding Problem for S.R. 29 West of Chaparral Slough*, PBS&J, October 1999. It is unknown when the pipe was replaced. The report, however, recommended double 72" pipes in this area. The double 54" pipes are undersized. They have the capacity for 135.04 cfs, however the runoff generated by the offsite area coming to the cross drain is 202.3 cfs for the 10 year design flood. A minimum of 2-66" pipes or pipes of equal capacity are needed to intercept this runoff.

### 5.0 **RECOMMENDATIONS**

This particular area of Glades County is consistently wet. The large offsite areas are extremely flat. The main causes of the flooding consist of heavy siltation, flat roadway profiles, undersized cross drains, and no distinct channel for water to flow. Runoff is essentially being constrained until a large event occurs which forces it to the outfall where it then floods the low point along the road.

The permanent solution to reduce flooding in this area is to raise the profile of the road and recreate the roadside ditches. However in order to provide an immediate and cost feasible solution to this problem, it is recommended to de-silt and dig out the cross drains, side drain, and adjacent ditches. Clearing the cross drains and the adjacent ditches within the right of way shall allow more unrestricted flow. Removal and replacement of the existing triple 30' cross drain pipes with quadruple 48" pipes will significantly improve the flooding. Water is currently backing up at this location and flooding the existing triple 36'' cross drain upstream since the existing triple 30'' cross drain is undersized. However, if this area is not maintained, it will silt up and flooding will still be a concern. In severe rain events the roadway may still overtop. This could have adverse impacts down stream however, if the flow is not maintained. The existing triple 36'' pipes are also recommended for replacement. This is mainly due to the fact that the cross drain contains the pipes from the original construction over 50 years ago. They were extended in 1977. The joints may be bad and the condition of the pipes is unknown. The pipes will remain the same size since the hydraulic analysis verified that the pipes have more than adequate capacity.

Therefore, the recommended temporary improvements to this area includes de-silting of the existing pipes and ditches, replacing the existing triple 30" pipes with quadruple 48" pipes at the first cross drain and replacement of the existing triple 36" pipes to the north due to failing joints.

This alternative may provide relief for the area, decreasing the potential for the road to flood until the facility is improved. This portion of S.R. 29 is included in the PD&E study that is currently being completed. This study may recommend that the roadway is raised which is the ultimate remedy to prevent flooding of the travel lanes.

Two preliminary cost estimates were developed using the FDOT Area 9 costs (see Appendix I). The first is for the proposed temporary solutions including upsizing the first cross drain,

replacing the second and regrading the ditches adjacent to the crossings. For this option, the estimated cost is \$143,300. The second cost estimate includes leaving both of the existing cross drains in place with desiliting and minor ditch regrading adjacent to the crossings. For this option, the estimated cost is \$11,500.
**Appendix A Original Drainage Complaint** 

# 0

#### Drainage Complaint - Inventory Data Sheet

#### SECTION I. LOCATION

County Glades

State Road SR 29

Reference No. 05090-1

Location - SR 29, north of SR 78 to Chaparral Slough

Road Description - 2-lane undivided roadway with shoulders and roadside ditches, with medium traffic volumes

Section/Township/Range Sec 36, T41S, R29E: Sec 1, 2, 11, 14, 15, T42S, R29E

#### SECTION II. PROBLEM DESCRIPTION

Problem: Flooding of right-of-way. Road does not flood.

How frequent does problem occur? 2 times in the past 10 years during the summer rainy season.

Estimate High Water Water is within 5 to 6 feet of roadway. High water is estimated to be at approximately 33.0 estimated from Historical Drainage Map (attached)

History of Problem High water in the right-of-way has occurred several times. Mr. Talbert Melton saw this section of roadway underwater in 1970.

Outfall description: Canals and natural tributaries

Persons Interviewed - FDOT personnel - Talbert Melton, Assistant Maintenance Engineer

### SECTION III. PROBLEM ANALYSIS

What is the cause of the flooding? Cause of flooding in 1970 is unknown. Natural ground in areas adjacent to SR 29 just north of SR 78 is shown on the quadrangle map at approx. elevation 35. Possibly, a cross drain could be damaged or crushed. Photographs of flooded areas show one side of the roadway ditch more flooded than the other.

Responsible entity for maintenance of the outfall: Unknown

What efforts have been made to fix the problems? FDOT cleans and maintains the side ditches

Damages or harm resulting from the flooding: Water in the shoulder area is not desirable.

### SECTION IV. PRELIMINARY CONCLUSIONS AND RECOMMENDATIONS

Monitor the area for future flooding events. Collect elevations at the roadway and side ditch profile elevations.



Drainage Complaint - Inventory Data Sheet

Reference No. 05090 - 1



SR 29 - Between Bridge 050033 and SR 78 on June 29, 1992 - wet season.



SR 29 - Between Bridge 050033 and SR 78 on June 29, 1992 - wet season.

Glades							
SR 29 - Roadway ID 05090 000		MP 4.709 - 6.877				Location A	
From Lone Pine Creek to	Cypress Bra	nch					
SFWMD Rainfall	Day	Rainfall	Duration	Rainfall	Frequency	1	
Gage Location	Aug. 18	0.17 in.	1 day	3.0 in.	10 Үеаг		
SR78	Aug. 19	3.44 in.	2 day	6.4 in.	25 Year		
	Aug. 20	2.95 in.	4 day	7.7 in.	10 Year		
	Aug. 21	1.14 in.				- <sup>-</sup>	

Maintenance reported roadway overtopping over this stretch of SR 29. During Drainage's field visit, water had reached the low member elevations of the Lone Pine Creek and Cypress Branch bridges. Water levels in the creeks matched the elevations of water in the roadside ditches and surrounding properties. Between Mile Post 5 and 6, water encroached up to 6 feet of the WB / SB travel lane, but no overtopping was observed. The roadside ditches were overtopped and surrounding properties were inundated with water. The bridges for Lone Pine Creek and Cypress Branch were observed to be free of debris and flowing full. The cross drains at M.P. 5.5 and 6.2 appeared to be free of debris. Water stages exceeded the headwall elevation at each culvert, and water flowing through the pipes with high velocities.

During this visit no damage to any drainage structures was visible at this location. Once water levels have receded, the drainage structures will be checked again by Drainage. The flooding issues appear to be caused by the low elevation of the roadway in relation to the surrounding land. To correct this issue the roadway would need to be raised. Maintenance should continue to post signs at any location where water is encroaching upon the roadway.

5.5- re-address second visit. soft spots. Water was high still. 6.2 - they may not have any problems.



































6100 6.237 (my Siso and Crac Drains

5229



Appendix B FDOT Straight Line Diagram





Appendix C FEMA Maps







Appendix D Correspondence

# Å A I M

# AIM Engineering & Surveying, Inc.

# **MEETING MINUTES**

Tampa Office 5802 Breckenridge Parkway Tampa, Florida 33610 813-627-4144 / Fax 813-627-1899 Toll Free: 888-627-4144

Lehigh Acres (Corporate Office) • Bartow • Tampa • Naples • Riviera Beach • Brooksville

Project:	Drainage Complaint Analysis Task #11 Project 1 and 2	
Subject:	Field Meeting	
Location:	SR 29 and SR 78 Date:	July 1, 2009
Attendees:	John Anderson (FDOT LaBelle Operations) Dawn Ratican (AIM)	
	Recorded By:	Dawn Ratican

SR 29 - between Lone Pine and Cypress Branch

- 0.5 miles of roadway under water
- Sheet flow wide open pasture both sides of road
- Channels downstream are not maintained
- Roadway shut down last year because of flooding
- Water stacks to the edge of the road on south side once a year
- 5-ft shoulders and resurfacing done 3-4 years ago. No money for pipe improvements.
- No planned projects for future
- At the culvert water is stacked up to shoulder (present) and water is stacked up to the fence in the south side.
- Both sides of roadway owned by Lykes Brothers
- No apparent flow, just a lot of standing water
- At Pine Fields water just sheet flows, nothing to restrict it.

SR 78

- 0.25 miles of roadway under water
- Lykes owns surrounding property
- 4-ft paved shoulders and pipes were replaced not upsized
- Ditches on both sides of roadway
- Right of way required for berm or dual ditch
- Deadman's branch north side own 300-ft no issues with drainage
- Cross drain no issues some ownership to the north
- Culvert at driveway flow from north around driveway
- Offsite single point discharge floods roadway

From: Anderson, John C [John.Anderson@dot.state.fl.us] Sent: Wednesday, July 29, 2009 7:50 AM To: Dawn Ratican Subject: RE: Drainage Complaints - Glades County SR 29 & SR 78

SR78: We have only seen the flooding in the area of the driveway with the side drain (no flooding at the bridges)

SR29: The flooding is between the two bridges (starting at the two bridges) (Lone Pine and Cypress) but it does get the worst at the two culverts and the worst of the two is the south culvert.

This past July 4<sup>th</sup> both areas flooded (SR78 and SR29). Like a dummy, I didn't take any pictures. I thought about it later, but I will try to find some old pictures of the area.

From: Dawn Ratican [mailto:dratican@aimengr.com]
Sent: Monday, July 27, 2009 10:37 AM
To: Anderson, John C
Subject: RE: Drainage Complaints - Glades County SR 29 & SR 78

Thank John. We were back out reviewing these projects last week. I wanted to verify that the only flooding on SR 78 is at the driveway with the side drain. Have you seen any flooding at the bridges? Also was this area flooded the July 4<sup>th</sup> weekend as well? On SR 29, does the flooding start at the culverts/bridges and then spread or occur in between the crossings? Do you have any photos of this most recent flooding?

Thanks again for your assistance. We will be wrapping up our review in the next couple of weeks.

Dawn

From: Anderson, John C [mailto:John.Anderson@dot.state.fl.us]
Sent: Wednesday, July 22, 2009 8:56 AM
To: Dawn Ratican
Subject: RE: Drainage Complaints - Glades County SR 29 & SR 78

Answers to your questions:

- All of our bridge information comes from District Bridge in Tampa. (Jose Garcia 813-744-6050 ext. 21227) (jose.garcia1@dot.state.fl.us)
- On SR29 the water flows from the west to the east. It will flow over the entire roadway (both lanes). Approximately a week after I meet with you (July 4<sup>th</sup> weekend) the road went under water due to heavy rains in the area, and yes almost every year this accurse.
- SR 78 is the same. SR 29 goes under water then approximately 8 to 10 hours later SR78 will go under water. SR78 also went under water the July 4<sup>th</sup> weekend.

From: Dawn Ratican [mailto:dratican@aimengr.com]
Sent: Tuesday, July 21, 2009 3:14 PM
To: Anderson, John C
Subject: Drainage Complaints - Glades County SR 29 & SR 78

Good afternoon John. I will be picking up the plans tomorrow that were copied for the SR 29 and SR 78 projects that we met on a few weeks back. I wanted to ask you a couple of more questions.

1. I am looking for the BHRs and Bridge Inspection Reports for all of the bridges. I believe that includes the Lone Pine Creek and Cypress Branch along SR 29 and Deadman's Branch and Cypress Branch along SR 78. Do you

have these or can you tell me who I should contact to obtain these?

2. I wanted to verify that during our discussions regarding the SR 29 flooding area, that water overtops the road from the downstream side (east side of road). I know that the road was shut down last year. Do you recall at what time of the year, also does this occur every year?

3. For the SR 78 flooding, when was the last time that the road overtopped? Again, is this a regular occurrence, or did the flooding take place during the construction of the shoulder improvements along this section?

Feel free to email me or call, whatever is most convenient for you. I will be heading down to Labelle tomorrow, so I can be reached on my cell phone (813-918-0280).

Thank you again for your help with these projects.

Dawn

Dawn Ratican, P.E. AIM Engineering & Surveying, Inc. 5802 Breckenridge Parkway, Suite 100 Tampa, Florida 33610 813.627.4144 (o) 813.918.0280 (c)

## **Dawn Ratican**

From:Anderson, John C [John.Anderson@dot.state.fl.us]Sent:Tuesday, November 24, 2009 3:17 PMTo:'dratican@aimengr.com'Subject:Fw: SR 29 and SR 78 Drainage Complaints

Sorry, it is in this one. It is the pipes at the cow pines.

John C. Anderson LaBelle Operation Center Telephone: 863-674-4027 Fax: 863-674-4030 Cell: 863-673-4056 E-Mail: john.anderson@dot.state.fl.us

From: McCormick, Steve To: Anderson, John C Sent: Tue Nov 24 10:52:34 2009 Subject: RE: SR 29 and SR 78 Drainage Complaints

The pipes at the cow pens appear to be 2-54" corrugated pipe, they are still under water. The next headwall north of there is 3-30" pipe and the one to the north of that is 3-36" pipe.

#### Steve

Steve McConnick Contracts Manager LaBelle Operation Center Florida Department Of Transportation (863)674-4027, Cell (863)673-4054 steve.mccormick@dot.state.fl.us

From: Anderson, John C Sent: Tuesday, November 24, 2009 8:27 AM To: McCormick, Steve Subject: Fw: SR 29 and SR 78 Drainage Complaints

Can you get me the size and number of pipes at these head walls on SR29. It is the two were it always floods. Thanks

John C. Anderson LaBelle Operation Center Telephone: 863-674-4027 Fax: 863-674-4030 Cell: 863-673-4056 E-Mail: john.anderson@dot.state.fl.us

From: Dawn Ratican
To: Anderson, John C
Sent: Mon Nov 23 10:14:31 2009
Subject: SR 29 and SR 78 Drainage Complaints
Good morning John. I wanted to see if you had an opportunity to review the draft reports we submitted to you for the SR 29 and SR 78 Drainage Complaints. We are finalizing the reports based on comments we received from FDOT D-1 Drainage Staff and want to include any revisions based on your comments.

12/9/2009

Also, we are trying to determine the size of the side drain pipe on the west side of SR 29 just north of Lone Pine Creek. In 1999 we know that there was one 30" pipe. Today there are two CMPs however due to the high elevation of standing water we were not able to determine the size. Do you know when these were replaced, who replaced them and what the current sizes are? Attached is a photo of the side drain.

Thanks for your assistance,

Dawn

# Dawn Ratican, P.E. AIM Engineering & Surveying, Inc.

5802 Breckenridge Parkway, Suite 100 Tampa, Florida 33610 813.627.4144 (o) 813.918.0280 (c) Appendix E Photos

# S.R. 29

# **Bridge Culvert at Lone Pine** Creek:



Looking south along S.R. 29 from the bridge culvert.



Ditch on southeast side of bridge.



Bridge structure number and name of water body.



East side of bridge.



Looking east from east side of bridge.



Ditch on northeast side of bridge.



Ditch on northwest side of bridge.



Looking west from west side of bridge.



West side of bridge.



Ditch on southwest side of bridge.



Water stains on west side of bridge.



Sidedrain at cow pens, just north of bridge on S.R. 29. Ditch almost at capacity.



Ditch on north side of driveway facing north.

# **Triple 30" Cross Drain:**



Looking south along S.R. 29 from cross drain.



Looking east from east side of cross drain.



Ditch on northeast side of cross drain.



Erosion at headwall on east side of cross drain.



Erosion at headwall on east side of cross drain.



Staining on headwall on east side of cross drain.



Ditch on northwest side of cross drain.


Looking west from headwall on west side of cross drain.



Ditch on southwest side of cross drain.



Headwall on west side of cross drain.



Water stains on headwall on west side of cross drain.

# Triple 36" Cross Drain:



Ditch on southeast side of cross drain.



Looking east from east side of cross drain.



Headwall on east side of cross drain.



Looking northeast from east side of cross drain.



Ditch on northeast side of cross drain.



Ditch on northwest side of cross drain.



Looking west from west side of cross drain.



Ditch on southwest side of cross drain.

## Bridge at Cypress Branch:



Bridge number.



Ditch on southeast side of bridge.



East side of bridge.



Looking east from east side of bridge.



Ditch on northeast side of bridge.



Ditch on northwest side of bridge.



Facing south along shoulder, west side of bridge.



West side of bridge.



Facing west from west side of bridge.



Facing southwest from west side of bridge. Trees appear to have water stains pretty high.



Ditch on southwest side of bridge.

Appendix F Drainage Maps





Appendix G Design Calculations

# SR 29

## Triple 30" Pipe

From Drainage Map: Total area is 216.9 Acres Project is located in Zone 8. Design Frequency is 50 years. Time of concentration is 3.7 hours. Area is grass and dirt and very flat, therefore the runoff coeficient is 0.3

## **Rational Method**

Q = CIA

Q: Flow Rate (cfs)
C: Runoff Coeficient
I: Rainfall Distribution (in/hr)
A: Area (acres)

Frequency (years)	l (in/hr)	Probability
25	1.45	
50	1.60	0.02
100	1.75	0.01
500		0.002

Q<sub>25</sub> = 0.3\*1.45\*216.9 Q<sub>25</sub> = 94.35 cfs

Q<sub>50</sub> = 0.3\*1.60\*216.9 Q<sub>50</sub> = 104.11 cfs

Q<sub>100</sub> = 0.3\*1.75\*216.9 Q<sub>100</sub> = 113.87 cfs

From graph:

Q<sub>500</sub> = 130 cfs

Figure 2, Example 1 taken from Drainage Handbook Culvert Design.



500 year design discharge (yellow) was found by drawing in the calculated Q values vs. the design frequency (red) and plotting a best fit line (green).



By: DMR Date: Check: Date: Revised: Date:

## Existing Conditions

## Triple 30" Cross Drain

Time of Concentration

Shee	t Flow					_
		Segmer	nt ID	AB		
1.	Surface description (table 3-1, TR-55)			Grass		
2.	Manning's Roughness coefficient, n (table 3-1, TR-55)			0.13		
З.	Two year 24 hour rainfall, P2		in	4.50		
4.	Flow length, L (total L< 300 ft)		ft	300		]
5.	Land slope , s	Begin Elev.	ft	35.00		
		End Elev.	ft	34.80		
	Slope = (E1-E2)/L	Slope	ft/ft	0.001		
6.	Tt = (0.007*(nL)^0.8)/((P2^0.5)(s^0.4)) * 60	Compute Tt	min.	69.2	+	= 69.2
Shall	ow Concentrated Flow			Par-		
	Grass	Segment ID		BD		
7.	Surface description (paved or unpaved)			Unpaved		
	Velocity Coefficient K (Paved = 20.328, Unpaved = 16	6.1345)		16.1345		
8.	Flow length, L		ft	3700		]
9.	Watercourse slope, s	Begin Elev.	ft	34.8		
		End Elev.	ft	32.5		
	Slope = (E1-E2)/L	Slope	ft/ft	0.001		
10.	Average velocity, V (V = K*S^0.5)		ft/s	0.40		
11.	Tt = L/(60*∨)	Compute Tt	min.	153.3	+	= 153.3
Char	nnel Flow (Ditch)					
		Segme	nt ID			
12.	Hydraulic radius, R = A / WP (Depth of Flow)		ft			
13.	Flow length, L		ft			
14.	Slope, s	Begin Elev.	ft			
		End Elev.	ft			
	Slope = (E1-E2)/L	Slope	ft/ft			
15.	Manning's roughness coefficient, N (table 3-1, TR-55)			]		
16.	V = (1.49*R^.67*s^0.5)/N		ft/s			
17.	$Tt=L/(60^*V)$	Compute Tt	min.			= 0.0
18.	Total of 6, 11 and 17		min.			= 222.5
	Minimum Time of Concentration		min.	10.0		

min. hr 222.5

# SR 29 – Existing Triple 30" Cross Drain

## **Project Notes**

Project Title: Drainage Complaint SR 29

Designer: EC

Project Date: Monday, August 03, 2009

Notes: The roadway overtops at a flow rate of 30.42 cfs. This is below the flow rate for the 25 year design frequency. The cross drain is undersized. All elevations were taken from old plans.

Table I Galling	ary of ourforthe	no at or oconing.	Inple de diedel	
Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
36.10	94.35	70.51	23.89	5
36.14	97.91	72.05	25.85	3
36.17	101.48	73.55	27.91	3
36.20	104.11	74.65	29.44	3
36.24	108.61	76.48	32.10	3
36.28	112.18	77.90	34.25	3
36.31	115.74	79.28	36.44	3
36.35	119.30	80.63	38.66	3
36.38	122.87	81.94	40.90	3
36.42	126.44	83.23	43.18	3
36.45	130.00	84.47	45.50	3

#### Table 1 - Summary of Culvert Flows at Crossing: Triple 30" Cross Drain

\*Highlighted values represent the 25 year, 50 year, and 500 year design frequencies.

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normai Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
94.35	70.51	36.10	2.600	6.021	4-FFf	2.500	1.645	2.500	5.220	4.788	0.000
97.91	72.05	36.14	2.641	6.056	4-FFf	2.500	1.664	2.500	5.220	4.892	0.000
101.48	73.55	36.17	2.682	6.092	4-FFf	2.500	1.682	2.500	5.220	4.994	0.000
104.11	74.65	36.20	2.712	6.118	4-FFf	2.500	1.696	2.500	5.220	5.069	0.000
108.61	76.48	36.24	2.763	6.163	4-FFf	2.500	1.718	2.500	5.220	5.194	0.000
112.18	77.90	36.28	2.803	6.198	4-FFf	2.500	1.736	2.500	5.220	5.290	0.000
115.74	79.28	36.31	2.843	6.233	4-FFf	2.500	1.752	2.500	5.220	5.383	0.000
119.30	80.63	36.35	2.882	6.267	4-FFf	2.500	1.766	2.500	5.220	5.475	0.000
122.87	81.94	36.38	2.920	6.302	4-FFf	2.500	1.779	2.500	5.220	5.564	0.000
126.44	83.23	36.42	2.958	6.336	4-FFf	2.500	1.792	2.500	5.220	5.652	0.000
130.00	84.47	36.45	2.996	6.370	4-FFf	2.500	1.804	2.500	5.220	5.736	0.000

#### Table 2 - Culvert Summary Table: Culvert 1

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

Inlet Elevation (invert): 30.08 ft, Outlet Elevation (invert): 30.08 ft

Culvert Length: 96.00 ft, Culvert Slope: 0.0000

\*



### Water Surface Profile Plot for Culvert: Culvert 1

### Site Data - Culvert 1

Site Data Option: Culvert Invert Data Inlet Station: 0.00 ft Inlet Elevation: 30.08 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Outlet Station: 96.00 ft (Straight Line Diagrams 05090000) Outlet Elevation: 30.08 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Number of Barrels: 3

#### **Culvert Data Summary - Culvert 1**

Barrel Shape: Circular Barrel Diameter: 2.50 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Inlet Type: Conventional Inlet Edge Condition: Square Edge with Headwall Inlet Depression: None

### Tailwater Channel Data - Triple 30" Cross Drain

Tailwater Channel Option: Enter Constant Tailwater Elevation Constant Tailwater Elevation: 35.30 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956)

## Roadway Data for Crossing: Triple 30" Cross Drain

Roadway Profile Shape: Constant Roadway Elevation Crest Length: 15.00 ft Crest Elevation: 35.45 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Roadway Surface: Paved Roadway Top Width: 24.00 ft

# SR 29

## Triple 36" Pipe

From Drainage Map: Total area is 123.1 Acres Project is located in Zone 8. Design Frequency is 50 years. Time of concentration is 2.7 hours. Area is grass and dirt and very flat, therefore the runoff coeficient is 0.3

## Rational Method

Q = CIA Q: Flow Rate (cfs) C: Runoff Coeficient I: Rainfall Distribution (in/hr) A: Area (acres)

Frequency (years)	l (in/hr)	Probability
25	1.80	
50	2.00	0.02
100	2.20	0.01
500		0.002

Q<sub>25</sub> = 0.3\*1.80\*123.1 Q<sub>25</sub> = 66.47 cfs

Q<sub>50</sub> = 0.3\*2.00\*123.1 Q<sub>50</sub> = 73.86 cfs

Q<sub>100</sub> = 0.3\*2.20\*123.1 Q<sub>100</sub> = 81.25 cfs

From graph:

Q<sub>500</sub> = 100 cfs

best fit line (green).

ത 500 year design discharge (yellow) was found by drawing in the calculated Q values vs. the design frequency (red) and plotting



Figure 2, Example 1 taken from Urainage Handbook Culvert Design.



By:	DMR	Date:
Check:		Date:
Revised:		Date:

## **Existing Conditions**

#### Sheet Flow

Surface description (table 3-1, TR-55)AB1.Surface description (table 3-1, TR-55)0.133.Two year 24 hour rainfall, P2in4.Flow length, L (total L< 300 ft)ft5.Land slope, sBegin Elev. ft5.Land slope, sEnd Elev. ft6.Tt = (0.007*(nL)*0.8)/((P2*0.5)(s*0.4))*60Compute Tt7.Surface description (paved or unpaved) Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345)BD9.Watercourse slope, sBegin Elev. ft9.Watercourse slope, sBegin Elev. ft9.Watercourse slope, sBegin Elev. ft9.Slope = (E1-E2)/LSlope ft/ft0.0010.00110.Average velocity, V (V = K*S^0.5)ft/s11.Tt = L/(60*V)Compute Tt12.Hydraulic radius, R = A / WP (Depth of Flow)ft13.Flow length, Lft14.Slope s (E1-E2)/LSlope15.Manning's roughness coefficient, N (table 3-1, TR-55)16.V = (1.49*R^A 67*s^0.5)/N17.Tt = L/(60*V)18.Total of 6, 11 and 17Minimum Time of Concentrationmin.hrhr	Sillee	L FIUW			2			-
1.       Surface description (table 3-1, TR-55)       Grass         2.       Manning's Roughness coefficient, n (table 3-1, TR-55)       in         3.       Two year 24 hour rainfall, P2       in         4.       Flow length, L (total L< 300 ft)			Segme	nt ID	AB		I	
2.       Manning's Roughness coefficient, n (table 3-1, TR-55)       0.13         3.       Two year 24 hour rainfall, P2       in         4.       Flow length, L (total L< 300 ft)	1.	Surface description (table 3-1, TR-55)			Grass			
3.       Two year 24 hour rainfall, P2       in       4.50         4.       Flow length, L (total L< 300 ft)	2.	Manning's Roughness coefficient, n (table 3-1, TR-55)			0.13	1	[	
4.       Flow length, L (total L< 300 ft)	3.	Two year 24 hour rainfall, P2		in	4.50	]	[	
5. Land slope , s Begin Elev. ft Slope = $(E1-E2)/L$ 6. Tt = $(0.007^{*}(nL)^{\circ}0.8)/((P2^{\circ}0.5)(s^{\circ}0.4))^{*} 60$ Shallow Concentrated Flow Grass 7. Surface description (paved or unpaved) Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345) 8. Flow length, L 9. Watercourse slope, s Flow length, L 10. Average velocity, V (V = K*S^{\circ}0.5) 11. Tt = L/(60^{*}V) Compute Tt 13. Flow length, L 14. Slope, s 14. Slope, s 15. Manning's roughness coefficient, N (table 3-1, TR-55) 16. V = $(1.49^{*}R^{\circ}.67^{*}s^{\circ}0.5)/N$ 17. Tt = L/(60^{*}V) 18. Total of 6, 11 and 17 Minimum Time of Concentration Time of Concentration N 10. Average velocity, V 10. Average roughness coefficient, N (table 3-1, TR-55) 16. V = $(1.49^{*}R^{\circ}.67^{*}s^{\circ}0.5)/N$ 17. Tt = L/(60^{*}V) 10. Average roughness coefficient, N (table 3-1, TR-55) 16. V = $(1.49^{*}R^{\circ}.67^{*}s^{\circ}0.5)/N$ 17. Tt = L/(60^{*}V) 18. Total of 6, 11 and 17 Minimum Time of Concentration Time of Concentration Nr	4.	Flow length, L (total L< 300 ft)		ft	300	]	ſ	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5.	Land slope , s	Begin Elev.	ft	35.00	]	ſ	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			End Elev.	ft	34.50	]	Γ	
6. $Tt = (0.007^*(nL)^{0.8})/((P2^{0.5})(s^{0.4})) * 60$ Compute $Tt$ min. $47.9$ + Shallow Concentrated Flow Grass Segment ID 7. Surface description (paved or unpaved) Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345) 8. Flow length, L ft 9. Watercourse slope, s Begin Elev. ft Slope = (E1-E2)/L Slope ft/ft 10. Average velocity, V (V = K*S^{0.5}) ft/s 11. $Tt = L/(60^*V)$ Compute $Tt$ min. $14.6$ + Channel Flow (Ditch) 12. Hydraulic radius, R = A / WP (Depth of Flow) ft 13. Flow length, L ft 14. Slope, s Begin Elev. ft Slope = (E1-E2)/L Slope ft/ft 15. Manning's roughness coefficient, N (table 3-1, TR-55) 16. $V = (1.49^*R^{-67}s^{*0.5})N$ ft/s 17. $Tt = L/(60^*V)$ Compute $Tt$ min. $10.0$ 18. Total of 6, 11 and 17 min. Minimum Time of Concentration min. $10.0$		Slope = (E1-E2)/L	Slope	ft/ft	0.002	]	E	
Shallow Concentrated Flow         Grass       Segment ID       BD         7.       Surface description (paved or unpaved)       Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345 )       If         8.       Flow length, L       ft       34.5         9.       Watercourse slope, s       Begin Elev. ft       32.9         10.       Average velocity, V (V = K*S^0.5)       ft/s       0.001         10.       Average velocity, V (V = K*S^0.5)       ft/s       0.39         11.       Tt = L/(60*V)       Compute Tt       min.         Channel Flow (Ditch)       ft       ft         12.       Hydraulic radius, R = A / WP (Depth of Flow)       ft         13.       Flow length, L       ft         14.       Slope = (E1-E2)/L       Slope fl/ft       ft         13.       Flow length, L       ft       ft         14.       Slope s       Begin Elev. ft       ft         I5.       Manning's roughness coefficient, N (table 3-1, TR-55)       ft       ft         16.       V = (1.49*R^{.67*s^{.0.5})/N       ft/s       ft         17.       Tt = L/(60*V)       Compute Tt       min.         18.       Total of 6, 11 and 17 <td>6.</td> <td><math>Tt = (0.007^{(nL)^{0.8})/((P2^{0.5})(s^{0.4})) * 60</math></td> <td>Compute Tt</td> <td>min.</td> <td>47.9</td> <td>] +</td> <td></td> <td></td>	6.	$Tt = (0.007^{(nL)^{0.8})/((P2^{0.5})(s^{0.4})) * 60$	Compute Tt	min.	47.9	] +		
GrassSegment IDBD7.Surface description (paved or unpaved) Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345)Unpaved8.Flow length, Lft9.Watercourse slope, sBegin Elev. ft34.5Slope = (E1-E2)/LSlope ft/ft10.Average velocity, V (V = K*S^0.5)ft/s11.Tt = L/(60*V)Compute Tt12.Hydraulic radius, R = A / WP (Depth of Flow)ft13.Flow length, Lft14.Slope = (E1-E2)/L15.Manning's roughness coefficient, N (table 3-1, TR-55)16.V = (1.49*R^.67*s^0.5)/N17.Tt = L/(60*V)18.Total of 6, 11 and 17 Minimum Time of Concentration Time of Concentrationmin.10.0hrNoning's multiple of Concentration min.18.Total of 6, 11 and 17 Minimum Time of Concentration Time of Concentration	Shall	ow Concentrated Flow						
7.       Surface description (paved or unpaved) Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345)       Unpaved 16.1345         8.       Flow length, L       ft         9.       Watercourse slope, s       Begin Elev. ft         Slope = (E1-E2)/L       Slope ft/ft       34.5         10.       Average velocity, V (V = K*S^0.5)       ft/s       0.39         11.       Tt = L/(60*V)       Compute Tt       min.         Channel Flow (Ditch)       Segment ID       114.6       +         12.       Hydraulic radius, R = A / WP (Depth of Flow)       ft       114.6       +         13.       Flow length, L       ft       114.6       +         14.       Slope, s       Begin Elev. ft       114.6       +         15.       Manning's roughness coefficient, N (table 3-1, TR-55)       16.       V = (1.49*R^A.67*s^0.5)/N       ft/s       11.         16.       V = (1.49*R^A.67*s^0.5)/N       ft/s       11.       10.0         18.       Total of 6, 11 and 17       min.       10.0       10.0         18.       Total of 6, 11 and 17       min.       10.0         18.       Total of Concentration       min.       10.0		Grass	Segment ID		BD		ĺ	
Velocity Coefficient K (Paved = 20.328, Unpaved = 16.1345 )16.13458.Flow length, Lft9.Watercourse slope, sBegin Elev. ftSlope = (E1-E2)/LSlope ft/ft0.00110.Average velocity, V (V = K*S^0.5)ft/s11.Tt = L/(60*V)Compute TtChannel Flow (Ditch)12.Hydraulic radius, R = A / WP (Depth of Flow)13.Flow length, Lft14.Slope, sBegin Elev. ft15.Manning's roughness coefficient, N (table 3-1, TR-55)ft/s16.V = (1.49*R^^67*s^0.5)/Nft/s17.Tt = L/(60*V)Compute Tt18.Total of 6, 11 and 17min.Minimum Time of Concentrationmin.ID.0.0Time of Concentration	7.	Surface description (paved or unpaved)			Unpaved			
8. Flow length, L ft 2700 9. Watercourse slope, s Begin Elev. ft 34.5 End Elev. ft 32.9 Slope = (E1-E2)/L Slope ft/ft 0.001 10. Average velocity, V (V = K*S^0.5) ft/s 0.39 11. Tt = L/(60*V) Compute Tt min. 114.6 + t Channel Flow (Ditch) 12. Hydraulic radius, R = A / WP (Depth of Flow) ft 13. Flow length, L ft 14. Slope, s Begin Elev. ft End Elev. ft End Elev. ft Slope = (E1-E2)/L Slope ft/ft 15. Manning's roughness coefficient, N (table 3-1, TR-55) 16. V = (1.49*R^^6.7*s^^0.5)/N ft/s 17. Tt = L/(60*V) Compute Tt min. 10.0 18. Total of 6, 11 and 17 min. Minimum Time of Concentration min. hr		Velocity Coefficient K (Paved = 20.328, Unpaved = 16	.1345)		16.1345			
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End Elev.ft $32.9$ Slope = (E1-E2)/LSlopeft/ft $0.001$ 10.Average velocity, V (V = K*S^0.5)ft/s $0.39$ 11.Tt = L/(60*V)Compute Ttmin.Channel Flow (Ditch)12.Hydraulic radius, R = A / WP (Depth of Flow)ft13.Flow length, Lft14.Slope, sBegin Elev.14.Slope, sBegin Elev.15.Manning's roughness coefficient, N (table 3-1, TR-55)ft/s16.V = (1.49*R^{A}.67*s^{0}.5)/Nft/s17.Tt = L/(60*V)Compute Tt18.Total of 6, 11 and 17min. min.Minimum Time of Concentrationmin. min.18.Total of 6, 11 and 17min. min.Minimum Time of Concentrationmin. min.hrhr	9.	Watercourse slope, s	Begin Elev.	ft	34.5		L	
Slope = (E1-E2)/LSlopeft/ft $0.001$ 10.Average velocity, V (V = K*S^0.5)ft/s $0.39$ 11.Tt = L/(60*V)Compute Ttmin.114.6+Channel Flow (Ditch)12.Hydraulic radius, R = A / WP (Depth of Flow)ft13.Flow length, Lft14.Slope, sBegin Elev.ft15.Manning's roughness coefficient, N (table 3-1, TR-55)ft/sImage: Compute Tt16.V = (1.49*R^0.67*s^0.5)/Nft/sft/s17.Tt = L/(60*V)Compute Ttmin.18.Total of 6, 11 and 17min.10.0Minimum Time of Concentrationmin.hr			End Elev.	ft	32.9		L	
10.Average velocity, V (V = K*S^0.5)ft/s0.3911.Tt = L/(60*V)Compute Ttmin.114.612.Hydraulic radius, R = A / WP (Depth of Flow)ft13.Flow length, Lft14.Slope, sBegin Elev.ft15.Manning's roughness coefficient, N (table 3-1, TR-55)ft/sImage: Slope ft/ft16.V = (1.49*R^.67*s^0.5)/Nft/sft/s17.Tt = L/(60*V)Compute Ttmin.18.Total of 6, 11 and 17min.10.0Time of Concentrationmin.hr		Slope = (E1-E2)/L	Slope	ft/ft	0.001		L	
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12.       Hydraulic radius, R = A / WP (Depth of Flow)       ft         13.       Flow length, L       ft         14.       Slope, s       Begin Elev.       ft         14.       Slope, s       Begin Elev.       ft         15.       Manning's roughness coefficient, N (table 3-1, TR-55)       Image: Slope ft/ft       Image: Slope ft/ft         16.       V = (1.49*R^{^{^{^{^{^{^{^{^{^{^{^{^{^{^{^{^{^{^{			Segme	nt ID				
13.Flow length, Lft14.Slope, sBegin Elev.ft14.Slope, sBegin Elev.ftEnd Elev.ftEnd Elev.ft15.Manning's roughness coefficient, N (table 3-1, TR-55)16.V = $(1.49*R^{A}.67*s^{A}0.5)/N$ ft/s16.V = $(1.49*R^{A}.67*s^{A}0.5)/N$ ft/s17.17.Tt = L/(60*V)Compute Ttmin.18.Total of 6, 11 and 17min.10.0Time of Concentrationmin.hr	12.	Hydraulic radius, R = A / WP (Depth of Flow)		ft			L	
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16.       V = (1.49*R^.67*s^0.5)/N       ft/s         17.       Tt = L/(60*V)       Compute Tt         18.       Total of 6, 11 and 17       min.         Minimum Time of Concentration       min.       10.0         Time of Concentration       min.         hr       hr	15.	Manning's roughness coefficient, N (table 3-1, TR-55)						_
17. Tt = L/(60*V)     Compute Tt     min.       18. Total of 6, 11 and 17     min.       Minimum Time of Concentration     min.       Time of Concentration     min.       hr     hr	16.	V = (1.49*R^.67*s^0.5)/N		ft/s				
18.     Total of 6, 11 and 17     min.       Minimum Time of Concentration     min.     10.0       Time of Concentration     min.       hr     hr	17.	$Tt = L/(60^*V)$	Compute Tt	min.		1		-
Minimum Time of Concentration min. 10.0 Time of Concentration min. hr	18.	Total of 6, 11 and 17		min.				
Time of Concentration min. hr		Minimum Time of Concentration		min.	10.0			
hr		Time of Concentration		min.				
				hr				

# SR 29 – Existing Triple 36" Cross Drain

## **Project Notes**

Project Title: Drainage Complaint SR 29

Designer: EC

Project Date: Monday, August 03, 2009

Notes: The roadway overtops at a flow rate of 193.86 cfs which is well beyond the flow rate for the 500 year design frequency. This cross drain seems to be sized properly. All elevations were taken from old plans.

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
33.81	66.47	66.47	0.00	1
33.85	69.82	69.82	0.00	1
33.88	73.18	73.18	0.00	1
33.89	73.86	73.86	0.00	1
33.96	79.88	79.88	0.00	1
34.00	83.23	83.23	0.00	1
34.04	86.59	86.59	0.00	1
34.08	89.94	89.94	0.00	1
34.12	93.29	93.29	0.00	1
34.17	96.65	96.65	0.00	1
34.22	100.00	100.00	0.00	1

#### Table 1 - Summary of Culvert Flows at Crossing: Triple 36" Cross Drain

\*Highlighted values represent the 25 year, 50 year, and 500 year design frequencies.

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
66.47	66.47	33.81	2.237	3.192	7-M1t	2.605	1.513	2.960	2.880	3.156	0.000
69.82	69.82	33.85	2.305	3.229	3-M2t	3.000	1.550	2.960	2.880	3.316	0.000
73.18	73.18	33.88	2.372	3.263	3-M2t	3.000	1.587	2.960	2.880	3.475	0.000
73.86	73.86	33.89	2.386	3.270	3-M2t	3.000	1.594	2.960	2.880	3.507	0.000
79.88	79.88	33.96	2.504	3.337	7-M2t	3.000	1.661	2.960	2.880	3.793	0.000
83.23	83.23	34.00	2.569	3.377	7-M2t	3.000	1.698	2.960	2.880	3.953	0.000
86.59	86.59	34.04	2.634	3.418	7-M2t	3.000	1.735	2.960	2.880	4.112	0.000
89.94	89.94	34.08	2.699	3.460	7-M2t	3.000	1.771	2.960	2.880	4.271	0.000
93.29	93.29	34.12	2.764	3.505	7-M2t	3.000	1.807	2.960	2.880	4.430	0.000
96.65	96.65	34.17	2.829	3.550	7-M2t	3.000	1.838	2.960	2.880	4.589	0.000
100.00	100.00	34.22	2.894	3.598	7-M2t	3.000	1.870	2.960	2.880	4.749	0.000

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Inlet Elevation (invert): 30.62 ft, Outlet Elevation (invert): 30.54 ft

Culvert Length: 92.00 ft, Culvert Slope: 0.0009

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## Water Surface Profile Plot for Culvert: Culvert 1

## Site Data - Culvert 1

Site Data Option: Culvert Invert Data Inlet Station: 0.00 ft Inlet Elevation: 30.62 ft Outlet Station: 92.00 ft Outlet Elevation: 30.54 ft Number of Barrels: 3

## **Culvert Data Summary - Culvert 1**

Barrel Shape: Circular Barrel Diameter: 3.00 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Inlet Type: Conventional Inlet Edge Condition: Square Edge with Headwall Inlet Depression: None

## Tailwater Channel Data - Triple 36" Cross Drain

Tailwater Channel Option: Enter Constant Tailwater Elevation Constant Tailwater Elevation: 33.50 ft (design highwater from historical plans)

## Roadway Data for Crossing: Triple 36" Cross Drain

Roadway Profile Shape: Constant Roadway Elevation Crest Length: 20.00 ft Crest Elevation: 36.20 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft

## SR 29 – Proposed Quadruple 48" Cross Drain

## **Project Notes**

Project Title: Drainage Complaint SR 29

Designer: EC

Project Date: Monday, August 03, 2009

Notes: The roadway would overtop at a flow rate of 113.3 cfs if this scenario were used. This falls between the 50 year and 100 year design floods and would be acceptable.

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
35.40	94.35	94.35	0.00	1
35.41	97.91	97.91	0.00	1
35.42	101.48	101.48	0.00	1
35.43	104.11	104.11	0.00	1
35.44	108.61	108.61	0.00	1
35.45	112.18	112.18	0.00	1
35.46	115.74	115.74	0.02	3
35.47	119.30	119.25	0.09	3
35.48	122.87	122.73	0.18	3
35.49	126.44	126.17	0.30	3
35.50	130.00	129.71	0.44	3

### Table 1 - Summary of Culvert Flows at Crossing: Quadruple 48" Cross Drain

\*Highlighted values represent the 25 year, 50 year, and 500 year design frequencies.

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Ou <b>tlet</b> Velocity (ft/s)	Tailwater Velocity (ft/s)
94.35	94.35	35.40	1.985	5.324	4-FFf	4.000	1.421	4.000	5.220	1.877	0.000
97.91	97.91	35.41	2.030	5.332	4-FFf	4.000	1.450	4.000	5.220	1.948	0.000
101.48	101.48	35.42	2.077	5.340	4-FFf	4.000	1.478	4.000	5.220	2.019	0.000
104.11	104.11	35.43	2.111	5.347	4-FFf	4.000	1.499	4.000	5.220	2.071	0.000
108.61	108.61	35.44	2.168	5.358	4-FFf	4.000	1.536	4.000	5.220	2.161	0.000
112.18	112.18	35.45	2.213	5.367	4-FFf	4.000	1.564	4.000	5.220	2.232	0.000
115.74	115.74	35.46	2.257	5.377	4-FFf	4.000	1.593	4.000	5.220	2.303	0.000
119.30	119.25	35.47	2.300	5.386	4-FFf	4.000	1.617	4.000	5.220	2.372	0.000
122.87	122.73	35.48	2.342	5.396	4-FFf	4.000	1.639	4.000	5.220	2.442	0.000
126.44	126.17	35.49	2.383	5.406	4-FFf	4.000	1.662	4.000	5.220	2.510	0.000
130.00	129.71	35.50	2.424	5.417	4-FFf	4.000	1.684	4.000	5.220	2.581	0.000

#### Table 2 - Culvert Summary Table: Culvert 1

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Inlet Elevation (invert): 30.08 ft, Outlet Elevation (invert): 30.08 ft

Culvert Length: 96.00 ft, Culvert Slope: 0.0000



## Water Surface Profile Plot for Culvert: Culvert 1

## Site Data - Culvert 1

Site Data Option: Culvert Invert Data Inlet Station: 0.00 ft Inlet Elevation: 30.08 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Outlet Station: 96.00 ft (Straight Line Diagrams 05090000) Outlet Elevation: 30.08 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Number of Barrels: 4

## **Culvert Data Summary - Culvert 1**

Barrel Shape: Circular Barrel Diameter: 4.00 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Inlet Type: Conventional Inlet Edge Condition: Square Edge with Headwall Inlet Depression: None

#### Tailwater Channel Data - Quadruple 48" Cross Drain

Tailwater Channel Option: Enter Constant Tailwater Elevation Constant Tailwater Elevation: 35.30 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956)

#### Roadway Data for Crossing: Quadruple 48" Cross Drain

Roadway Profile Shape: Constant Roadway Elevation Crest Length: 15.00 ft Crest Elevation: 35.45 ft (SR 78, Job No. 0504-201, Glades County, Fiscal Year 1956) Roadway Surface: Paved Roadway Top Width: 24.00 ft

## SR 29

Project is located in Zone 8. Area is grass and dirt and very flat, therefore the runoff coeficient is 0.3 Areas were taken from the drainage map

#### **Rational Method**

Existing flow rates generated from runoff Q = CIAQ: Flow Rate (cfs) C: Runoff Coeficient, <u>0.3</u> I: Rainfall Distribution (in/hr), <u>1.1</u> A: Area (acres), <u>612.9</u>

#### 54" Side drain pipe

Q<sub>10</sub> = 0.3\*1.1\*612.9 Q<sub>10</sub> = 202.3 cfs

## Manning's Equation

Side drain pipe capacity  $Q = 1.49/n (A)(R)^{2/3}(S)^{1/2}$ Q: Flow Rate (cfs) n: Roughness Coefficient A: Area of Pipe (ft<sup>2</sup>) R: Hydraulic Raduis (ft) S: Slope of Pipe

Pipe Size	Area, A	Perimeter, WP	Hydraulic Radius, R	n	S	Q (per Barrel)
54.00	15.90	14.13	1.13	0.012	0.001	67.52
60.00	19.63	15.70	1.25	0.012	0.001	89.42
66.00	23.75	17.27	1.38	0.012	0.001	115.29

\*existing pipe size is bolded



Project: SR 29 Subject: Time of Concentration, Tc

By:	DMR	Date:
Check:		Date:
Revised:		Date:

## Existing Conditions Double 54" Side Drain

Sheet	Flow				2 3	
		Segmer	nt ID	AB		
1.	Surface description (table 3-1, TR-55)			Grass		
2.	Manning's Roughness coefficient, n (table 3-1, TR-55)			0.13		
3.	Two year 24 hour rainfall, P2		in	4.50		
4.	Flow length, L (total L< 300 ft)		ft	300		
5.	Land slope , s	Begin Elev.	ft	48.00		
		End Elev.	ft	46.00		
	Slope = (E1-E2)/L	Slope	ft/ft	0.007		
6.	Tt = (0.007*(nL)^0.8)/((P2^0.5)(s^0.4)) * 60	Compute Tt	min.	27.5	+	= 27.5
Shallo	ow Concentrated Flow					1
	Grass	Segment ID		BD		
7.	Surface description (paved or unpaved)			Unpaved		
	Velocity Coefficient K (Paved = 20.328, Unpaved = 16	5.1345)		16.1345		
8.	Flow length, L		ft	8300		
9.	Watercourse slope, s	Begin Elev.	ft	46.0		
		End Elev.	ft	35.0		
	Slope = (E1-E2)/L	Slope	ft/ft	0.001		
10.	Average velocity, V (V = K*S^0.5)		ft/s	0.59		
11.	Tt = L/(60*∨)	Compute Tt	min.	235.5	+	= 235.5
Chan	nel Flow (Ditch)					
		Segmei	nt ID			
12.	Hydraulic radius, R = A / WP (Depth of Flow)		ft			
13.	Flow length, L		ft			
14.	Slope, s	Begin Elev.	ft			
		End Elev.	ft			
	Slope = (E1-E2)/L	Slope	ft/ft			
15.	Manning's roughness coefficient, N (table 3-1, TR-55)					
16.	V = (1.49*R^.67*s^0.5)/N		ft/s			0
17.	Tt = L/(60*V)	Compute Tt	min.			= 0.0
18.	Total of 6, 11 and 17		min.			= 263.1
	Minimum Time of Concentration		min.	10.0		
	Time of Concentration		min.			263.1
			hr			4.38

Appendix H Analysis of Flooding Report

## ANALYSIS OF FLOODING PROBLEM FOR S.R. 29 WEST OF CHAPARRAL SLOUGH

05090 CO. Glade CO.

Financial Project ID 198356-1-32-04 Work Program Item No. 1119960 S.R. 29 between S.R. 78 and Chaparral Slough

**Prepared** for:

The Florida Department of Transportation District One 801 North Broadway Bartow, FL 33831-12349

**Prepared by:** 

PBS&J, Inc. 5300 W. Cypress Street Suite 300 Tampa, FL 33607-1066

October 1999

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Lang Dardy 10/24/99

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POTENTIAL CA	USES OF FLOODING	
RECOMMENDA	TIONS	
FIGURE	TITLE	FOLLOWS PAGE
1	EXISTING DRAINAGE PATTERNS	1

## APPENDICES

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- APPENDIX APhotographs of FloodingAPPENDIX BFlooding Correspondence
- AT LIGHT D THOUS CONSpondence
- APPENDIX C Hydraulic Calculations
- APPENDIX D Drainage Maps



## INTRODUCTION

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The project is located in Glades County, just east of the City of Labelle. It is a two lane rural highway constructed in the late 1940's. See Appendix D for the original drainage map, dated 1947 and a drainage map for SR 78 dated 1956 which includes this section of roadway. As noted on the original map, the section of roadway between Lone Pine Creek Bridge and 600 ft west of Chaparral Slough Bridge (between Stations 366+00 and 426+00) is flat, with a profile elevation of 35.4. This section of roadway has flooded twice since sometime in the early 1970's, see Appendix B for flooding correspondence. See Appendix A for photographs of a flooding event on March 20, 1998 and a near flood event on February 18, 1998.

The intent of this engineering analysis is to determine the cause of the flooding and provide recommendations regarding the most cost effective remedy.

## **EXISTING DRAINAGE PATTERNS**

Drainage is generally toward the south and consists of shallow sloughs, marshes, and wetlands. Because the terrain is so flat, flow is at low velocity and the difference between flood stages and normal wet season water levels is only a few feet. There is significant storage within the basins. Drainage structures within and near the flooding area consist of a 45' bridge at Lone Pine Creek, a 150' bridge at Chaparral Slough, (3) - 30" cross drain pipes at approximately Sta. 354+00, (3) - 36" cross drain pipes at approximately Sta. 392+00, and a 30" side drain pipe under a drive approximately 700 ft northeast of Lone Pine Creek Bridge. All structures except the side drain and (3) - 36" pipe were constructed on the original project. Time of construction and reason for the (3) - 36" pipes are unknown.

The original roadway project provided a large R/W ditch on the northwest side of SR 29 between Lone Pine Creek Bridge and a slough approximately 1500 ft northeast of the bridge. It connected the slough, which drains a 2618 acre basin (see original project drainage map), with Lone Pine Creek Bridge. The old plans show the R/W ditch begins at station 315+70.50 and ends at approximately 325+00. The ditch has a 20 ft bottom width and a 0.15% profile grade (Elv 28.65 @ Sta 316+00 to Elv 30.00 @ Sta 325+00).

During flood conditions, a large area northwest of SR 29 stores runoff and rises to elevation 35.4 before overtopping occurs. There is a slight gradient on the pool due to its movement toward the southwest.

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## POTENTIAL CAUSES OF FLOODING

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A review of the area drainage was performed using the following sources:

Two field reviews. (August 26, 1999 and September 7, 1999) Drainage maps for SR 29, Project 05090-1 (date 1947) and SR 78, Project 05040- (date 1956) USGS Quadrangle maps Photographs of flood events. Interviews with Talbert Melton (FDOT Maintenance - Labelle), George Vialas (Engineer for Lykes Bros.), Joseph Phillips (FDOT SWAO Drainage Engineer) District One Flood Inventory, 1996 Bridge Hydraulic Report for Lone Pine Creek Bridge, by JMI Engineers, March 7, 1996 Aerial reconnaissance, August 29, 1999

Potential causes and their evaluations are listed below:

- The flow capacity of Chaparral Slough Bridge could be significantly reduced by partial blockage resulting from accumulation of Water Hyacinths. This could cause diversion of flow to the southwest and overload the pipes and Lone Pine Creek bridge. A review of the photograph taken at Chaparral Slough bridge during flooding conditions on February 18, 1998 shows low velocities through the bridge, thus significant head losses would not be possible.
- The large R/W ditch northeast of Lone Pine Creek Bridge is restricted by a 30" side drain pipe under a driveway, approximately 700 ft from the bridge. This restriction could be contributing to the flooding problem in two ways. It could increase the head losses, which will cause immediate and direct impacts to flood stages and/or it could indirectly raise flood stages by raising the seasonal high waters, thus reducing available storage. This could be a significant problem, depending on the ratio between the flow carried in the R/W ditch and that carried overland in shallow depth flow. It is obvious that the 30" pipe violates the design intent of the R/W ditch, since its capacity is so much less than the ditch.
- The water elevation observed on August 26, 1999 at Chaparral Slough was approximately 1 ft below the bottom slab of the bridge. This is estimated to be

elevation 34.0, which is the same elevation of the highwater shown on the old drainage map. Since no significant rainfall had occurred within several days it suggest the seasonal high water stages downstream of Chaparral Slough Bridge may have been raised through land alterations. This would raise flood elevations for two reasons, bridge hydraulics and the loss of storage. No land alterations south of the bridge were observed during aerial reconnaissance.

- Chaparral Slough may overtop its western basin boundary during flood stages and increase flow to the (3)-30", (3)-36", and Lone Pine Creek Bridge above that anticipated in the original design. This diversion of flow could be natural or caused by land alteration. Survey efforts and comprehensive modeling will be required to determine if this is occurring, however it is not warranted at this time. It is obvious that the discharge to the pipes and Lone Pine Creek Bridge exceed their capacity, regardless of source.
- The location of the (3)-30" pipes offers a more efficient path for removal of flood waters than either Lone Pine Creek or Chaparral Slough. This is apparent from the photographs of flooding. It is obvious that the tailwater stages at this site are 2' to 3' below upstream stages. Low velocities at the bridge, observed during flood events, indicate losses through the bridges are low. If the headwaters are near 35.5, as shown in the photograph, then the tailwaters at the bridges are much closer to the 35.5 than at the pipes. An examination of the USGS Quadrangle map adds further support to the hypothesis that tailwater stages are lower at the pipes than at the two bridges. Based on these facts, the original design may not have been the most hydraulically effective. Clearly, the most certain remedy for the flood condition is a significant enlargement of the (3)-30" pipes, however such alteration can have far reaching impacts. The increased discharge rate downstream would require concurrence of property owners and the Water Management District.

#### RECOMMENDATIONS

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Improvements should be implemented in a two step process as follows:

• Replace the 30" side drain pipe in the R/W ditch at approximately Sta 320+22 with 40 ft of double 72" pipes at approximately elevation 28.8. This will provide capacity

which conforms to the original design intent at a relatively economical expense, especially if FDOT Maintenance employees perform the work. This improvement will reduce flood levels in two ways; (1) it will reduce head losses through the side drain and (2) it will draw down water levels more rapidly which, in turn, will increase storage available, and thus will lower flood stages. Benefits provided by this action are unknown without a significant surveying and modeling effort since they depend on the ditch conveyance as compared to overland conveyance. It is more cost effective to invest in the pipe than in the study to predict their benefits.

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Step two is to be implemented if the side drain enlargement does not reduce flooding to an acceptable level. It requires replacement of the (3)-30" cross drain pipes with a box culvert. This modification will involve design, permitting, and possible acquisition of flood rights from downstream property owners.

If Step Two cannot be implemented, two options are available, both of which are significantly more expensive:

• Option One requires raising the roadway grade for approximately 2 miles, extending cross drains, and adding 48" cross drains uniformly spaced along the raised segment. Weirs with crest elevations to match existing roadway elevation (35.4) will be affixed to the 48" cross drains to serve as control structures to match existing conditions, i.e. restricted pipe flow up to 35.4 and a large capacity increase above 35.4. This modification can be designed to provide stage-discharge characteristics similar to existing overtopping conditions, thus maintaining existing flow conditions. There are significant construction cost attached to this option.

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Option Two will provide a raised berm along the northwest R/W line at elevation 35.4 with openings equivalent to existing pipes. Several 48" cross drains will be provided under the roadway to carry the large berm overtopping flow and distribute it along the southeast R\W line in a manner similar to what happens during existing roadway overtopping. Construction cost will be less than Option One, however it will require a R\W strip along the northwest side in order to provide sufficient width to construct the berm and ditches (one on either side of the berm). Option Two will be more economical than option one because R/W acquisition will involve undeveloped property in one ownership.

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## APPENDIX A

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## PHOTOGRAPHS OF FLOODING



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LOOKING NORTHEAST ALONG S.R. 29 AT CHAPARRAL SLOUGH BRIDGE



LOOKING SOUTH ALONG S.R. 29 AT (3) 30" RCP APPROXIMATELY STA. 354+00


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CDN 3-1--39

NUMER DRAWARE SP. 11 GUARDS CD - FIN.

LOOKING SOUTHWEST ALONG S.R. 29 BETWEEN LONE PINE CREEK AND CHAPARRAL SLOUGH



LOOKING SOUTHWEST ALONG S.R. 29 BETWEEN LONE PINE CREEK AND CHAPARRAL SLOUGH



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LOOKING NORTHEAST ALONG S.R. 29 BETWEEN LONE PINE CREEK AND CHAPARRAL SLOUGH



LOOKING NORTHEAST ALONG S.R. 29 BETWEEN LONE PINE CREEK AND CHAPARRAL SLOUGH

#### **APPENDIX B**

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### FLOODING CORRESPONDENCE

DATE:	<u>02/07/96</u>	<b>PROJECT NO: </b> <u>9523</u>
TIME:	10:29 AM	CALL PLACED/RECEIVED BY: Paula
FIRM C	ALLED: FDOT Maintena	TELEPHONE # (941) 674-4027
SPOKE	WITH: Talbert Melton	

Subject: Flooding on SR29

I asked Mr. Melton specifically about June 1995 when Ken Howard recalls there was a need for barricades on a portion of SR 29 where water was coming onto the roadway. He does not remember ever having to take barricades out there. He said that the water frequently comes up and will quickly runoff the roadway. At times they have gone out and driven fluorescent painted stakes at the edge of the pavement, however they have not had to drive stakes in a while.

He also spoke with field superintendent Robert Crawford who would actually gone out into the field. Mr. Crawford does not remember water over the road or bridges. He did not take barricades out during this event.

The other field superintendent, Wally Thalen, was out of the office but will call when he gets in.

Project/Proposal

CC:

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File, Dave, Art.

9523

DATE: 02/07/96

PROJECT NO: 9523

TIME: 11:40 AM CALL PLACED/RECEIVED BY: Paula

FIRM CALLED: FDOT Maintenance

**TELEPHONE #** (941) 674-4027

SPOKE WITH: Wallace Thalen

Subject: Flooding on SR29

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The area of flooding during June 1995 was at a 36" cross drain located between bridges 050033 and 050035. It is approximately 0.5 - 0.6 miles south of bridge 050033. This is the area that they have the most problems with. During June the water was up to the edge of pavement. It lacked only a few inches to overtop the road. You could not pull off the highway.

Water flows 'real good' through bridge 050035. It washes sand up on the east side of the highway. He does not remember the water level ever coming up to the bridge.

Project/Proposal

File, Dave, Art

9523

DATE: 02/07/96

#### **PROJECT NO: 9523**

TIME: 09:23 AM

#### CALL PLACED/RECEIVED BY: Paula

FIRM CALLED: Glades Co.

**TELEPHONE #** (941) 946-1217

Emergency Management

SPOKE WITH: Ken Howard, Director

Subject: Flooding on SR29

I called Mr. Howard about the flooding which occurred around June 23, 1995 (per Art de Laski). He stated that the road was never completely closed during this time. There was an area where the water was over the road, however the road was still passible. A Florida Highway Patrol first noticed the water on the road and notified the EMA, who notified the Glades Co. Road Department who put up barricades and warnings for travelors.

Mr. Howard stated that this area was located about 5 miles south of the intersection of SR 29 and US 27. He said that it was not at a bridge, it was only the roadway. The water receded in about 24 hours.

Mr. Howard stated that all this information was his own personal experience. This past year was unusual due to several tropical storms, hurricanes, etc. and there were a lot of areas flooded which usually don't. In the last 7-10 years, he does not remember SR 29 ever overtopping. The EMA does not have detailed records of flooding and road closures. Since this is a state road, he recommended contacting the FDOT Maintenance.

He mentioned contacting Tommy Greenwood, Director of the Glades County Road Department for possibly more information. (941) 946-0771

Project/Proposel

File, Dave, Art

cc:

DATE: 02/23/96

#### PROJECT NO: 9523

#### TIME: 09:11 AM CALL PLACED/RECEIVED BY: Paula

FIRM CALLED: Glades Co. Schools TELEPHONE # (941) 946-0323 ext. 13

SPOKE WITH: Norman (Sonny) Hughes, Dir. of Transportation

Subject: Flooding on SR 29

Mr. Hughes has been with the Glades Co. School Department for 29 years. Glades Co. School buses travel SR 29 from LaBelle to Palmdale and are not allowed to drive on roads which have water overtopping them. He said that during the period of time he has been with Glades Co., SR 29 has never been blocked for the school buses. He said that there have been other roads which have been blocked but not SR 29.

Project/Proposal

cc:

FILE. DFS

9523

DATE: 02/23/96

**PROJECT NO: <u>9523</u>** 

TIME: 10:01 AM

CALL PLACED/RECEIVED BY: Paula

FIRM CALLED: Glades Co.

TELEPHONE # (941) 946-0533

SPOKE WITH: Jerry Harris. Building Director

Subject: Flooding on SR 29

Mr. Harris is the former Glades Co. Emergency Management Director (1978-1995). He also has been the FEMA Flood Program Administrator since 1982. He was born and raised in Clewiston and considers himself a "Sawgrass Mugrat".

Speaking with Mr. Harris about flooding on SR 29, he mentioned that the only location where they have had trouble on this road is at Chaparral Slough. He recalls that the water has come up very high at this location, enough to damage the roadway base, but has not overtopped the roadway.

He said that during heavy rains water will spread out on both sides of SR 29, and sheetflow across the floodplain approximately 200 square miles. He said that all the water in this area is trying to reach the Caloosahatchee River regardless of the direction it travels.

**Project/Proposal** 

cc:

FILE. DFS

9523

 DATE:
 02/26/96
 PROJECT NO: 2523

 TIME:
 11:59 AM
 CALL PLACED/RECEIVED BY: Paula

FIRM CALLED: Glades Co.

TELEPHONE # (941) 675-0124

SPOKE WITH: David Whiddon. Former Road Department Superintendent

Subject: Flooding on SR29

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Mr. Whiddon was with the road department from 1980-93, prior to Tommy Greenwood. He has lived in Glades County all his life, 48 years.

To his knowledge, SR 29 has never overtopped. He stated that the land to the west of SR 29, north of SR 78 approximately 3-4 miles stays wet for most of the year.

He warned that if the flow was increased through SR 29, this could cause increased flooding at SR 78. He said that the residents on Marshall Field Road get mad every year because of flooding. If we increase the risk of flooding for these residents, he said for us to expect a lawsuit.

He said that during heavy rains, the water already comes up to the edge of pavement on SR 78.

Project/Proposal

9523

File, Dave, Art

CC:

#### APPENDIX C

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### HYDRAULIC CALCULATIONS



COMP. BY:	MDM			
CHK. BY:		E	2	
DATE:	9	14	99	
SHEET NO:				
JOB NO:				

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157	. j - 4	eak Basin Dise		3.454
	USGS Regress Regi	tion Equations. on A	FHWA Regress Zon	on Equations, e1
Storm (yr.)	Flow (m <sup>3</sup> /m)	Flow (cfs)	Flow (m <sup>3</sup> /m)	Flow (cfs)
2	234	138	514	· 303
50	1015	598	2138	1258
100	1164	685	2498	1470
500	1485	874	N/A -	N/A

Table 1. Peak Basin Discharge

The resistance to flow, Manning's "n" coefficients, in the main channel and the flood plain have been calculated using procedures and equations found in the <u>Guide for Selecting Manning's</u> <u>Roughness Coefficients for Natural Channels and Flood Plains</u>, FHWA-TS-84-204. Very high amounts of vegetation, a severe degree of irregularity, and a negligible effect of obstructions in the main channel are factors which effect the resistance to flow. A Manning's Roughness Coefficient of 0.10 was used to account for this resistance to flow in the main channel of Lone Pine Creek (calculations provided in Appendix B).

The Manning's Roughness Coefficients for the flood plain were computed without using the vegetation-density method. Since the roughness is not uniformly distributed across the flood plain it has been subdivided into two sections. These sections include an area with trees and area of pasture (no trees). The computed "n" value for the pasture section is 0.06 and 0.15 for the section with trees (calculations and photographs provided in Appendix B).

#### Hydraulic Analysis

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FHWA's Bridge Waterways Analysis Model (WSPRO) was used to create a hydraulic model of Lone Pine Creek at the crossing of SR 29.

SR - 29 Lone Pine Creek Bridge No. 050035

#### APPENDIX D

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## DRAINAGE MAPS

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Appendix I Cost Estimate



#### AIM ENGINEERING & SURVEYING, INC. 5802 Breckenridge Prky. Tampa, Florida 33610 (813) 627-4144

#### ENGINEER'S ESTIMATE OF CONSTRUCTION COST

Project Name: SR 29 From North of SR 78 to Chaparral Slough (M.P 4.709 to 6.877) - Ditch Regrading						
County: Glades County						
Engineer of Record: Dawn Ratican, P.E. AIM Project No: 09-9662						
Type of Estimate: Preliminary Planning (X) Phase I () Phase	e II ( ) Phase III ( ) Phase IV ( ) Final ( )					
Estimated By: Liz Cashwell	Date: 08/26/09 Spec Year:					
Checked By: Dawn Ratican	Date: 08/28/09					

	Estimated Cost	Percent of Total Cost
COMPONENT GROUPS		
200 - ROADWAY	\$ 7,930.65	100.00%
		ļ
COMPONENT SUB-TOTAL	\$ 7,930.65	100.00%
(101-1) MOBILIZATION (10%)	\$ 793.07	10.00%
(102-1) MAINTENANCE OF TRAFFIC (10%)	\$ 793.07	10.00%
(999-25) CONTINGENCY (Do Not Bid) (25%)	\$ 1,982.66	25.00%
PROJECT GRAND TOTAL =	\$ 11,499.44	

NOTES : Unit costs were determined from previously bid unit cost averages of Area 9 and Statewide.

Pay Item Number	Item Description	Estimated Quantity	Unit	Unit Cost	Estimated Cost
	ROADWAY				
101-1	Mobilization	1.0	LS	\$0.00	\$ -
102-1	Maintenance of Traffic	1.0	LS	\$0.00	\$ -
104-11	Floating Turbidity Barrier	100.0	LF	\$5.73	\$ 573.00
104-12	Staked Turbidity Barrier	50.0	LF	\$5.97	\$ 298.50
104-13-1	Silt Fence Staked (Type III)	400.0	LF	\$7.60	\$ 3,040.00
110-1-1	Clearing & Grubbing	0.75	AC	\$551.96	\$ 413.97
120-1	Excavation, Regular	350	CY	\$1.81	\$ 633.50
430-94-2	Desilt Pipe (25" - 36")	656	LF	\$4.53	\$ 2,971.68
430-94-4	Desilt Pipe (49" - 60")	100.0	LF	\$10.60	\$ 1,060.00
570-1-2	Turf Complete (Sodding)	3,630	SY	\$1.31	\$ 4,755.30
				Subtotal =	\$ 7,930.65



## AIM ENGINEERING & SURVEYING, INC. 5802 Breckenridge Prky. Tampa, Florida 33610 (813) 627-4144

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#### ENGINEER'S ESTIMATE OF CONSTRUCTION COST \_

Project Name: SR 29 From North of SR 78 to Chaparral Slough (M.P 4.709 to 6.877) - Culvert Replacement						
County: Glades County						
Engineer of Record: Dawn Ratican, P.E. AIM Project No: 09-9662						
Type of Estimate: Preliminary Planning (X) Phase I ()	Phase II ( ) Phase III ( ) Phase IV ( ) Final ( )					
Estimated By: Liz Cashwell	Date: 08/26/09 Spec Year:					
Checked By: Dawn Ratican	Date: 08/28/09					

	Estimated Cost	Percent of Total Cost
COMPONENT GROUPS		
200 - ROADWAY	\$ 98,798.51	100.00%
COMPONENT SUB-TOTAL	\$ 98,798.51	100.00%
(101-1) MOBILIZATION (10%)	\$ 9,879.85	10.00%
(102-1) MAINTENANCE OF TRAFFIC (10%)	\$ 9,879.85	10.00%
(999-25) CONTINGENCY (Do Not Bid) (25%)	\$ 24,699.63	25.00%
PROJECT GRAND TOTAL =	\$ 143,257.84	

Unit costs were determined from previously bid unit cost averages of Area 9 and Statewide. NOTES :

Pay Item Number	Item Description	Estimated Quantity	Unit	Unit Cost		Estimated Cost	
	ROADWAY						
101-1	Mobilization	1.0	LS	\$0.00	\$		
102-1	Maintenance of Traffic	1.0	LS	\$0.00	\$	-	
104-11	Floating Turbidity Barrier	100.0	LF	\$5.73	\$	573.00	
104-12	Staked Turbidity Barrier	50.0	LF	\$5.97	\$	298.50	
104-13-1	Silt Fence Staked (Type III)	400.0	LF	\$0.76	\$	304.00	
110-1-1	Clearing & Grubbing	0.75	AC	\$551.96	\$	413.97	
120-1	Excavation, Regular	350	CY	\$1.81	\$	633.50	
400-1-2	Conc Class I (Endwalls)	43.6	CY	\$773.16	\$	33,725.24	
430-175-102	Pipe Culv (Opt Matl) (Round 25" to 36" S/CD)	276.0	LF	\$69.23	\$	19,107.48	
430-175-103	Pipe Culv (Opt Matl) (Round 37 to 48" S/CD)	384	LF	\$101.53	\$	38,987.52	
570-1-2	Turf Complete (Sodding)	3,630	SY	\$1.31	\$	4,755.30	
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				Subtotal =	\$	98,798.51	

# APPENDIX I Existing Typical Section



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T: PROJECTS VENVIRONMENTAL DI DW Permil and MII\Task 12 Drainage Improvements Construction Plans SR29\_GladesCounty \4313941520\ roadway \yps rd01.dgn 2:35:04 PM

TYPICAL SECTION

SHEET NO. 3

# APPENDIX J Historical Rainfall Data

ZONES FOR PRECIPITATION IDF CURVES DEVELOPED BY THE DEPARTMENT







Displaying April, 2018 Monthly Normal Precipitation Valid on: May 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying August, 2018 Monthly Normal Precipitation Valid on: September 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying December, 2018 Monthly Normal Precipitation Valid on: January 01, 2019 12:00 UTC What is UTC time? Map Help





Displaying February, 2022 Monthly Normal Precipitation Valid on: March 01, 2022 12:00 UTC What is UTC time? Map Help





Displaying January, 2022 Monthly Normal Precipitation Valid on: February 01, 2022 12:00 UTC What is UTC time? Map Help





Displaying July, 2018 Monthly Normal Precipitation Valid on: August 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying June, 2018 Monthly Normal Precipitation Valid on: July 01, 2018 12:00 UTC What is UTC time? Map Help




Displaying March, 2018 Monthly Normal Precipitation Valid on: April 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying May, 2018 Monthly Normal Precipitation Valid on: June 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying November, 2018 Monthly Normal Precipitation Valid on: December 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying October, 2018 Monthly Normal Precipitation Valid on: November 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying September, 2018 Monthly Normal Precipitation Valid on: October 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying 2015 Annual Observed Precipitation Valid on: January 01, 2016 12:00 UTC What is UTC time? Map Help





Displaying 2016 Annual Observed Precipitation Valid on: January 01, 2017 12:00 UTC What is UTC time? Map Help





Displaying 2017 Annual Observed Precipitation Valid on: January 01, 2018 12:00 UTC What is UTC time? Map Help





Displaying 2018 Annual Observed Precipitation Valid on: January 01, 2019 12:00 UTC What is UTC time? Map Help





Displaying 2019 Annual Observed Precipitation Valid on: January 01, 2020 12:00 UTC What is UTC time? Map Help





Displaying 2020 Annual Observed Precipitation Valid on: January 01, 2021 12:00 UTC What is UTC time? Map Help





Displaying 2021 Annual Observed Precipitation Valid on: January 01, 2022 12:00 UTC What is UTC time? Map Help





Displaying 2015 Annual Normal Precipitation Valid on: January 01, 2016 12:00 UTC What is UTC time? Map Help

# APPENDIX K Hydraulic Modeling Data



Faller, Davis & Abbociates, Inc. 5555 W. CYPRESS ST. - SUITE 300 CERTIFICATE OF AUTORIZATION NO.1 5864 DEBORAH L KNICHTON, P.E. NO.1 49920

DEPARTMENT OF TRANSPORTATION									
ROAD NO.	COUNTY	FINANCIAL PROJECT ID							
SR 29	GLADES	417878-1-22-01							

DRAINAGE AREA CROSSDRAIN AT STA. 356+00 SHEET NO.



By:	DMR	Date:
Check:		Date:
Revised:		Date:

## Existing Conditions Triple 30" Cross Drain

Shee	t Flow				_	_	
		Segme	nt ID	AB		]	
1.	Surface description (table 3-1, TR-55)			Grass			
2.	Manning's Roughness coefficient, n (table 3-1, TR-55)			0.13			
3.	Two year 24 hour rainfall, P2		in	4.50			
4.	Flow length, L (total L< 300 ft)		ft	300			
5.	Land slope , s	Begin Elev.	ft	35.00			
		End Elev.	ft	34.80			
	Slope = (E1-E2)/L	Slope	ft/ft	0.001		] _	
6.	Tt = (0.007*(nL)^0.8)/((P2^0.5)(s^0.4)) * 60	Compute Tt	min.	69.2	] +	] = [	69.2
Shall	ow Concentrated Flow						
	Grass	Segment ID		BD		]	
7.	Surface description (paved or unpaved)			Unpaved	1	1	
	Velocity Coefficient K (Paved = 20.328, Unpaved = 16	6.1345)		16.1345	1	1	
8.	Flow length, L		ft	3700	1	1	
9.	Watercourse slope, s	Begin Elev.	ft	34.8	1	1	
		End Elev.	ft	32.5	1	1	
	Slope = (E1-E2)/L	Slope	ft/ft	0.001	1	1	
10.	Average velocity, V (V = K*S^0.5)		ft/s	0.40			
11.	Tt = L/(60*V)	Compute Tt	min.	153.3	] +	] = [	153.3
Char	nel Flow (Ditch)						
		Segme	nt ID		1	1	
12.	Hydraulic radius, R = A / WP (Depth of Flow)		ft		1	1	
13.	Flow length, L		ft		1	1	
14.	Slope, s	Begin Elev.	ft		1	1	
		End Elev.	ft		1	1	
	Slope = (E1-E2)/L	Slope	ft/ft		1	1	
15.	Manning's roughness coefficient, N (table 3-1, TR-55)				1	1	
16.	V = (1.49*R^.67*s^0.5)/N		ft/s		1	1	
17.	Tt = L/(60*V)	Compute Tt	min.		]	] = [	0.0
18.	Total of 6, 11 and 17		min.			=	222.5
	Minimum Time of Concentration		min.	10.0			
	Time of Concentration		min.				222.5
			hr				3.71
	Minimum Time of Concentration Time of Concentration		min. min. hr	10.0		E	222 3.7





## Crossing - Existing S-1 (Sta 356+00), Design Discharge - 145.0 cfs Culvert - Triple 30" Cross Drain, Culvert Discharge - 80.4 cfs

#### Site Data – Existing 30" Triple Cross Drain

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 29.80 ft (As-builts from FPID 431394-1-52-01)

Outlet Station: 96.00 ft (As-builts from FPID 431394-1-52-01)

Outlet Elevation: 28.70 ft (As-builts from FPID 431394-1-52-01)

Number of Barrels: 3

## Culvert Data Summary – Existing 30" Triple Cross Drain

Barrel Shape: Circular Barrel Diameter: 2.50 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Culvert Type: Straight Inlet Configuration: Square Edge with Headwall (Ke=0.5)

## Tailwater Channel Data – Existing 30" Triple Cross Drain

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 35.30 (SR 78, Job No 0504-201, Glades County, Fiscal Year 1956)

## Roadway Data for Crossing – Existing 30" Triple Cross Drain

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 21.50 ft

Crest Elevation: 35.35 ft (As-builts from FPID 431394-1-52-01)

Roadway Surface: Paved

Roadway Top Width: 34.00 ft

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	Triple 30" Cross Drain Discharge (cfs)	Roadway Discharge (cfs)	Iterations
36.34	50 year	145.00	80.45	64.52	4

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Normal Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
50 year	145.00	80.45	36.34	1.34	2.50	6.60	5.46	0.00





### Site Data – Proposed 42" Triple Cross Drain, No Profile Rise

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 29.80 ft (As-builts from FPID 431394-1-52-01)

Outlet Station: 96.00 ft (As-builts from FPID 431394-1-52-01)

Outlet Elevation: 28.70 ft (As-builts from FPID 431394-1-52-01)

Number of Barrels: 3

#### Culvert Data Summary – Proposed 42" Triple Cross Drain, No Profile Rise

Barrel Shape: Circular Barrel Diameter: 3.50 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Culvert Type: Straight Inlet Configuration: Square Edge with Headwall (Ke=0.5)

## Tailwater Channel Data – Proposed 42" Triple Cross Drain, No Profile Rise

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 35.30 (SR 78, Job No 0504-201, Glades County, Fiscal Year 1956)

## Roadway Data for Crossing – Proposed 42" Triple Cross Drain, No Profile Rise

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.83 ft

Crest Elevation: 35.35 ft (As-builts from FPID 431394-1-52-01)

Roadway Surface: Paved

Roadway Top Width: 34.00 ft

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	Triple 42" Cross Drain Discharge (cfs)	Roadway Discharge (cfs)	Iterations
35.81	50 year	145.00	117.23	27.85	7

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Normal Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
50 year	145.00	117.23	35.81	1.39	3.50	6.60	4.06	0.00



Crossing - Proposed S-1, 2-ft Profile Raise (Sta 356+00), Design Discharge - 145.0 cfs Culvert - Triple 42" Cross Drain, Culvert Discharge - 145.0 cfs

### Site Data – Proposed 42" Triple Cross Drain, 2-ft Profile Rise

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 29.80 ft (As-builts from FPID 431394-1-52-01)

Outlet Station: 96.00 ft (As-builts from FPID 431394-1-52-01)

Outlet Elevation: 28.70 ft (As-builts from FPID 431394-1-52-01)

Number of Barrels: 3

## Culvert Data Summary – Proposed 42" Triple Cross Drain, 2-ft Profile Rise

Barrel Shape: Circular Barrel Diameter: 3.50 ft Barrel Material: Concrete Barrel Manning's n: 0.0120 Culvert Type: Straight Inlet Configuration: Square Edge with Headwall (Ke=0.5)

## Tailwater Channel Data – Proposed 42" Triple Cross Drain, 2-ft Profile Rise

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 35.30 (SR 78, Job No 0504-201, Glades County, Fiscal Year 1956)

## Roadway Data for Crossing – Proposed 42" Triple Cross Drain, 2-ft Profile Rise

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.83 ft

Crest Elevation: 37.35 ft

Roadway Surface: Paved

Roadway Top Width: 34.00 ft

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	Triple 42" Cross Drain Discharge (cfs)	Roadway Discharge (cfs)	Iterations
36.08	50 year	145.00	145.00	0.00	1

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Normal Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
50 year	145.00	145.00	36.08	1.57	3.50	6.60	5.02	0.00
## APPENDIX L Detour Map

## SR 29 Detour Route Map

