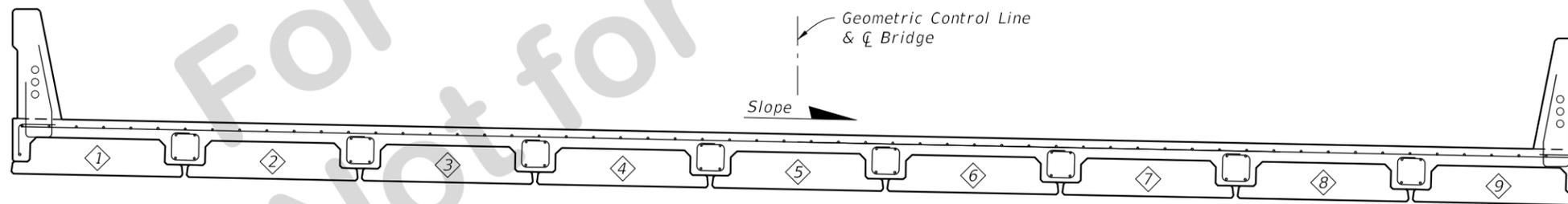


TYPICAL SECTION THRU SUPERSTRUCTURE WITH CROWN



TYPICAL SECTION THRU SUPERSTRUCTURE WITH CONSTANT CROSS SLOPE

(SEE SECTION WITH CROWN ABOVE FOR ALL INFORMATION NOT SHOWN IN THIS SECTION)

- NOTES:
1. See Structure Plans General Notes for Topping minimum thickness and cover.
 2. Work this Index with Index D400-300.

SUPERSTRUCTURE TYPICAL SECTION

LAST REVISION
01/01/26

REVISION

DESCRIPTION:

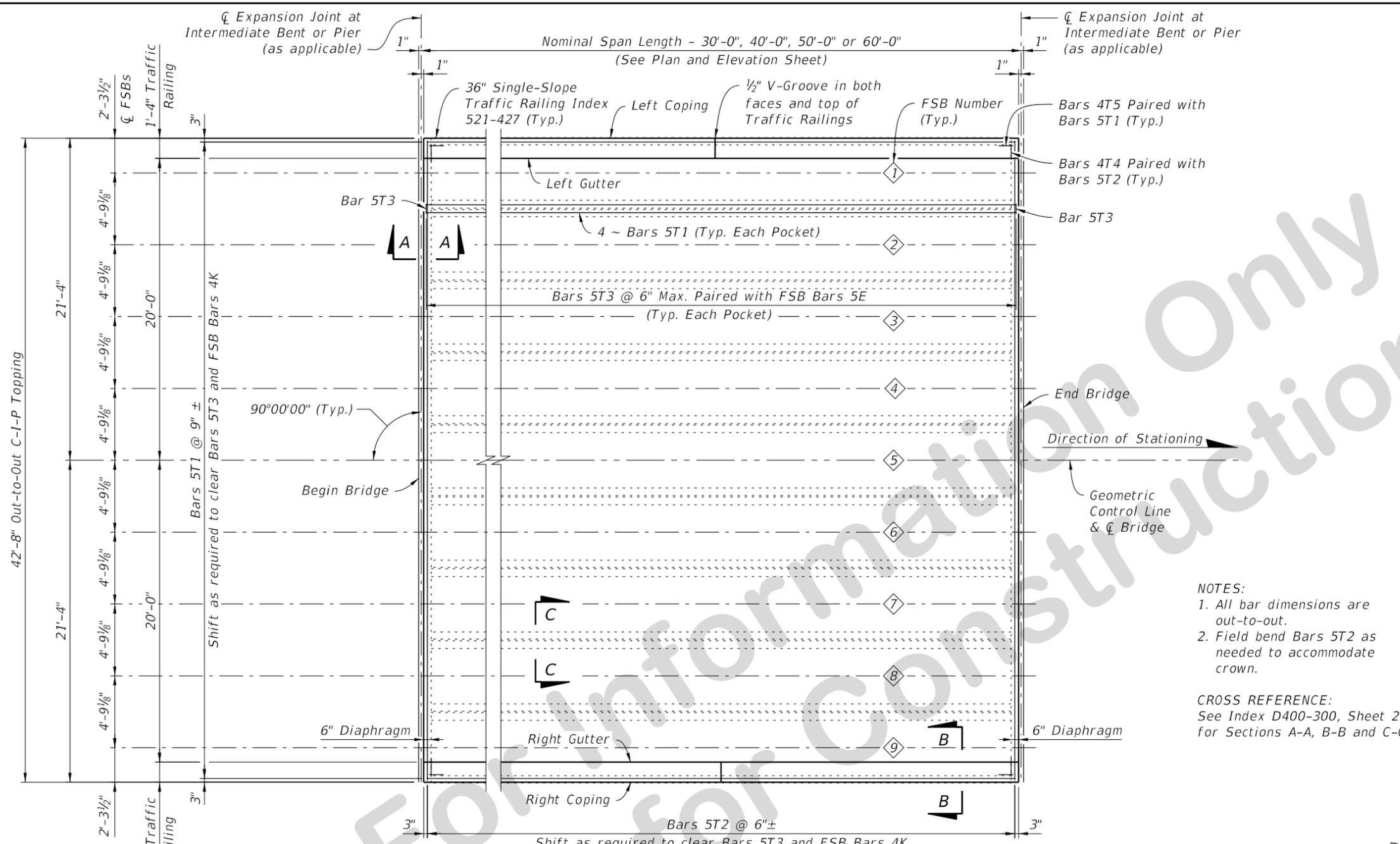


DEVELOPMENTAL
STANDARD PLANS

FSB SUPERSTRUCTURE PACKAGE
40 FT. CLEAR WIDTH

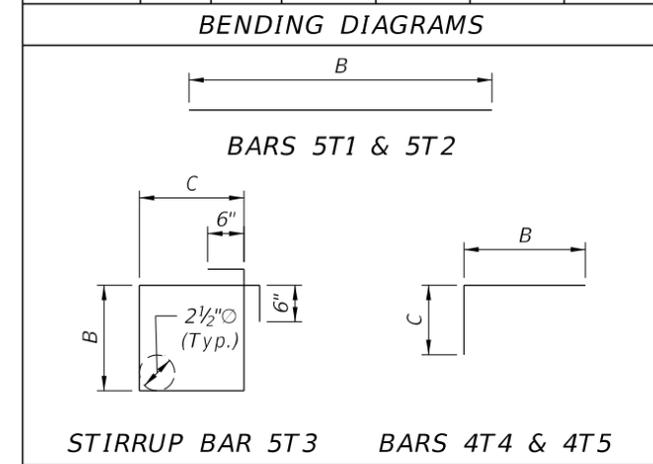
INDEX
D400-340

SHEET
1 of 5



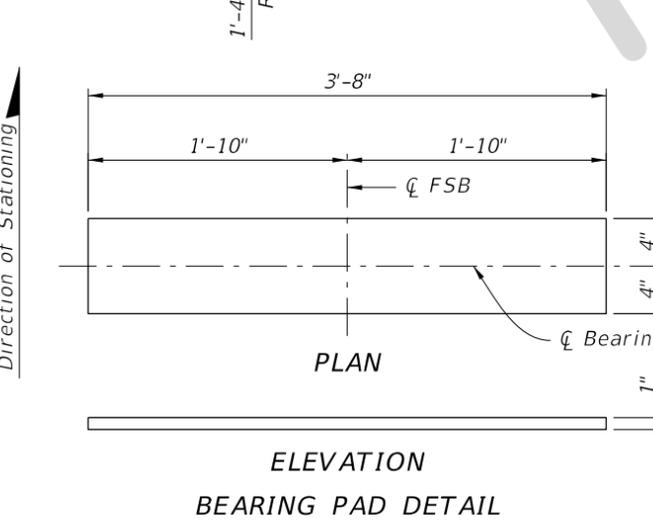
BILL OF REINFORCING STEEL FOR ONE SPAN ONLY

| NOMINAL SPAN LENGTH | MARK | | LENGTH | NO. OF BARS | B | C |
|---------------------|------|-----|--------|-------------|--------|-------|
| | SIZE | DES | | | | |
| 30'-0" | 5 | T1 | 29'-6" | 92 | 29'-6" | |
| | 5 | T2 | 42'-4" | 62 | 42'-4" | |
| | 5 | T3 | 4'-2" | 480 | 9½" | 9½" |
| | 4 | T4 | 2'-2" | 120 | 1'-6" | 8" |
| | 4 | T5 | 2'-6" | 116 | 1'-6" | 1'-0" |
| 40'-0" | 5 | T1 | 39'-6" | 92 | 39'-6" | |
| | 5 | T2 | 42'-4" | 82 | 42'-4" | |
| | 5 | T3 | 4'-2" | 640 | 9½" | 9½" |
| | 4 | T4 | 2'-2" | 160 | 1'-6" | 8" |
| | 4 | T5 | 2'-6" | 116 | 1'-6" | 1'-0" |
| 50'-0" | 5 | T1 | 49'-6" | 92 | 49'-6" | |
| | 5 | T2 | 42'-4" | 102 | 42'-4" | |
| | 5 | T3 | 4'-7" | 800 | 1'-0" | 9½" |
| | 4 | T4 | 2'-5" | 200 | 1'-6" | 11" |
| | 4 | T5 | 2'-9" | 116 | 1'-6" | 1'-3" |
| 60'-0" | 5 | T1 | 59'-6" | 92 | 59'-6" | |
| | 5 | T2 | 42'-4" | 122 | 42'-4" | |
| | 5 | T3 | 5'-1" | 960 | 1'-3" | 9½" |
| | 4 | T4 | 2'-8" | 240 | 1'-6" | 1'-2" |
| | 4 | T5 | 3'-0" | 116 | 1'-6" | 1'-6" |



NOTES:
 1. All bar dimensions are out-to-out.
 2. Field bend Bars 5T2 as needed to accommodate crown.

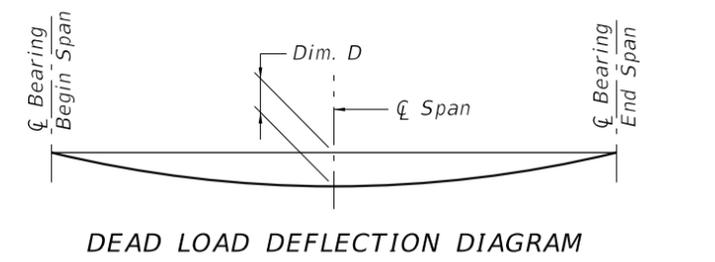
CROSS REFERENCE:
 See Index D400-300, Sheet 2 for Sections A-A, B-B and C-C.



Note:
 Provide Plain Neoprene Bearing Pads in accordance with Specification 932. Use a Shear Modulus of:
 (1) G = 150 psi for 18" FSBs
 (2) G = 110 psi for both 12" FSBs and 15" FSBs.

FLORIDA SLAB BEAM - CAMBER AND DEFLECTION DATA TABLE

| SPAN LENGTH (ft.) | BEAM NO. | NET BEAM CAMBER (PRESTRESS - DEAD LOAD OF BEAM) @ 120 DAYS (in.) | DIM D DEAD LOAD DEFLECTION DUE TO TOPPING POUR @ 120 DAYS (in.) |
|-------------------|----------|--|---|
| 30 | 1-9 | ½" | ¾" |
| 40 | 1-9 | 1 5/16" | 5/8" |
| 50 | 1-9 | 1 3/8" | 7/8" |
| 60 | 1-9 | 1 11/16" | 1 1/8" |



CAMBER NOTE:
 The values given in the table are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the topping casting. If the predicted cambers based on field measurements differ more than ± ½" from the theoretical "Net Beam Camber @ 120 Days" shown in the table, propose modified dimensions as required and submit to the Engineer for approval a minimum of 21 days prior to casting topping concrete.

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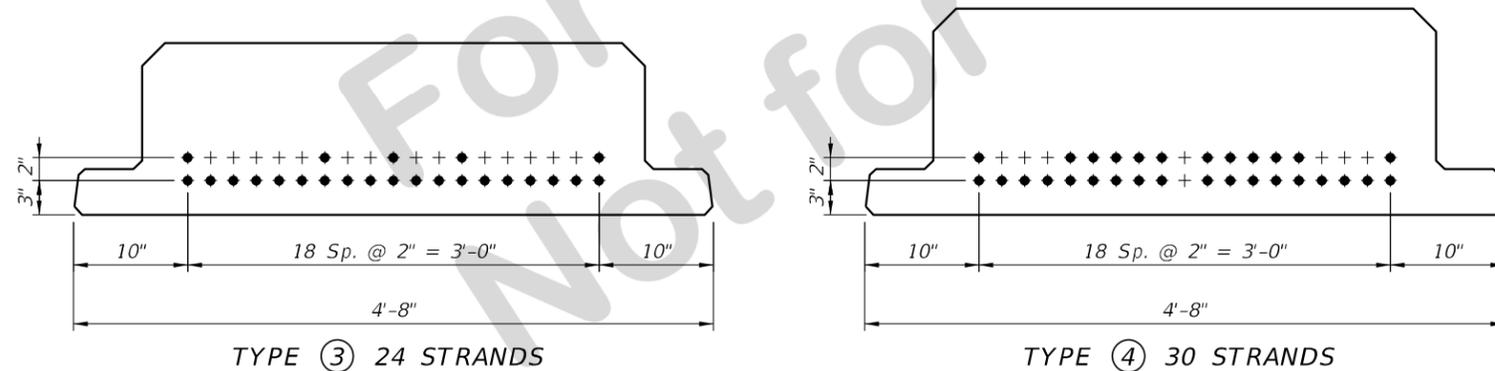
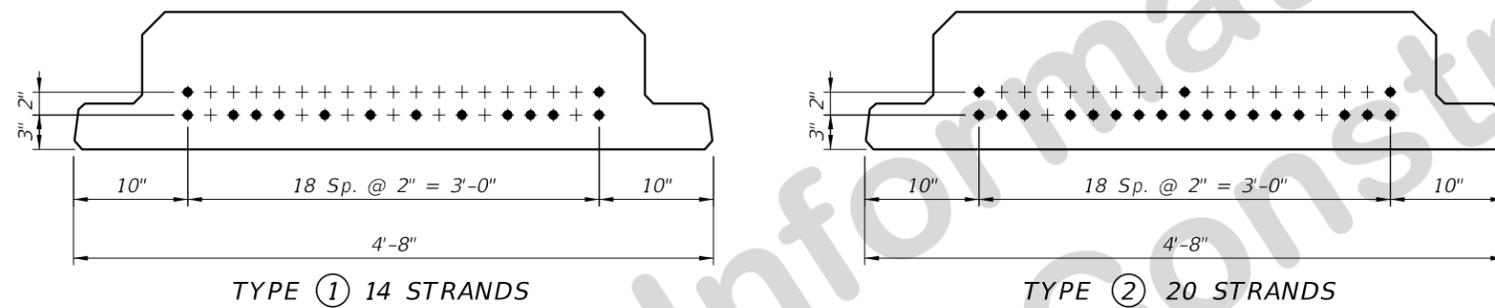
FLORIDA SLAB BEAM - TABLE OF VARIABLES

| NOMINAL SPAN LENGTH | LOCATION BEAM NO. | BEAM TYPE | CONCRETE PROPERTIES | | | | STND. PTRN. TYPE | PLAN VIEW CASE | | ANGLE θ | | BEAM DIMENSIONS* | | | REINFORCING STEEL | | | | | | | | | | | | | | | | | | | | |
|---------------------|-------------------|-----------|---------------------|-----------------|---------|-------|------------------|----------------|-------|----------------|-------|------------------------|-----------------|-----|----------------------|-------|-------|-----|-------|-----|-------|-------|----------------------|----------------------|----------------------|-----|----|-----|-------|-------|--|-------------------|--|--------------|--|
| | | | CLASS | STRENGTHS (psi) | | END 1 | | END 2 | END 1 | END 2 | DIM W | DIM L | DIM R | 3C | | 4D1 | | 4D2 | | 4D3 | | 5E1 | | 5E2 | | 6Y1 | | 6Y2 | | 4K | | NO. OF BAR SPACES | | BAR SPACING* | |
| | | | | 28 Day | Release | | | | | | | | | NO. | DIM C | DIM D | DIM D | NO. | DIM E | NO. | DIM E | DIM Y | DIM Y | NO. | S1 | S2 | V1 | V2 | | | | | | | |
| 30' | 1,9 | FSB 12x56 | VI | 8500 | 6800 | 1 | 1 | 1 | 90° | 90° | 4'-8" | 28'-10 $\frac{1}{4}$ " | $\frac{1}{4}$ " | 31 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 24 | 2'-4" | - | - | 57 | 4'-4 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 84 | 1 | 8 | 1'-6" | 1'-6" | | | | | |
| | 2-8 | FSB 12x56 | VI | 8500 | 6800 | 1 | 1 | 1 | 90° | 90° | 4'-8" | 28'-10 $\frac{1}{4}$ " | $\frac{1}{4}$ " | 31 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 24 | 2'-4" | 57 | 4'-6" | - | - | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 84 | 1 | 8 | 1'-6" | 1'-6" | | | | | |
| 40' | 1,9 | FSB 12x56 | VI | 8500 | 6800 | 2 | 1 | 1 | 90° | 90° | 4'-8" | 38'-10 $\frac{3}{8}$ " | $\frac{3}{8}$ " | 41 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 32 | 2'-4" | - | - | 77 | 4'-4 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 156 | 8 | 10 | 6" | 1'-6" | | | | | |
| | 2-8 | FSB 12x56 | VI | 8500 | 6800 | 2 | 1 | 1 | 90° | 90° | 4'-8" | 38'-10 $\frac{3}{8}$ " | $\frac{3}{8}$ " | 41 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 32 | 2'-4" | 77 | 4'-6" | - | - | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 156 | 8 | 10 | 6" | 1'-6" | | | | | |
| 50' | 1,9 | FSB 15x56 | VI | 8500 | 6800 | 3 | 1 | 1 | 90° | 90° | 4'-8" | 48'-10 $\frac{1}{2}$ " | $\frac{1}{2}$ " | 51 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 44 | 2'-4" | - | - | 97 | 4'-4 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 148 | 4 | 13 | 1'-0" | 1'-6" | | | | | |
| | 2-8 | FSB 15x56 | VI | 8500 | 6800 | 3 | 1 | 1 | 90° | 90° | 4'-8" | 48'-10 $\frac{1}{2}$ " | $\frac{1}{2}$ " | 51 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 44 | 2'-4" | 97 | 4'-6" | - | - | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 148 | 4 | 13 | 1'-0" | 1'-6" | | | | | |
| 60' | 1,9 | FSB 18x56 | VI | 8500 | 6800 | 4 | 1 | 1 | 90° | 90° | 4'-8" | 58'-10 $\frac{5}{8}$ " | $\frac{5}{8}$ " | 61 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 52 | 2'-4" | - | - | 117 | 4'-4 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 224 | 12 | 15 | 6" | 1'-6" | | | | | |
| | 2-8 | FSB 18x56 | VI | 8500 | 6800 | 4 | 1 | 1 | 90° | 90° | 4'-8" | 58'-10 $\frac{5}{8}$ " | $\frac{5}{8}$ " | 61 | 4'-3 $\frac{1}{2}$ " | 2'-4" | 2'-4" | 52 | 2'-4" | 117 | 4'-6" | - | - | 3'-3 $\frac{1}{2}$ " | 3'-3 $\frac{1}{2}$ " | 224 | 12 | 15 | 6" | 1'-6" | | | | | |

NOTE: Work this sheet with Standard Plans Index 450-450.

DIMENSION NOTES

* All longitudinal slab beam dimensions shown on this sheet with a single asterisk (*) are measured along the top of beam at the centerline. Dimension "R" is calculated at mid-height of the slab beam.



STRAND PATTERNS

STRAND DESCRIPTION: Use 0.60" Diameter, Grade 270, Low Relaxation Carbon Steel Strands stressed at 44.0 kips each. Area per strand equals 0.217 sq. in.

STRAND DEBONDING LEGEND

• - fully bonded strands.

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FLORIDA SLAB BEAM - TABLE OF VARIABLES

| | | | | | | | |
|---------------------------|----------|--------------|--|------------------------------|--|-------------------|-----------------|
| LAST REVISION 01/01/26 | REVISION | DESCRIPTION: | | DEVELOPMENTAL STANDARD PLANS | FSB SUPERSTRUCTURE PACKAGE 40 FT. CLEAR WIDTH | INDEX D400-340 | SHEET 3 of 5 |
|---------------------------|----------|--------------|--|------------------------------|--|-------------------|-----------------|

Table 2 - LRFR - Load Rating Summary Details for Prestressed Concrete Bridges (Flat Slab and Deck/Girder)

Superstructure - Environmental Classification: Slightly/Moderately Aggressive

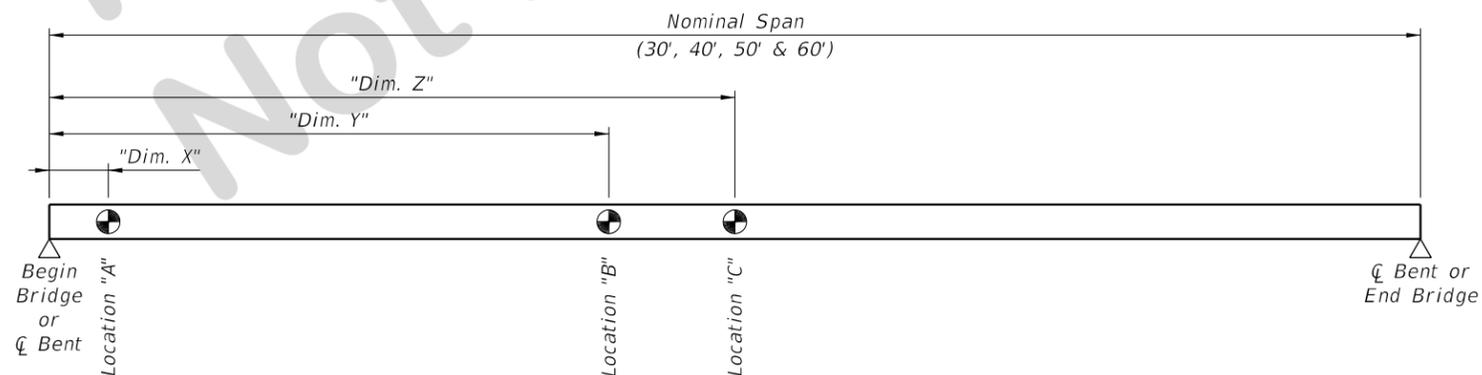
| Span Length (Ft) | Level | Limit State | Vehicle | Weight (tons) | Load Factors | | | Moment (Strength) or Stress (Service) | | | | | Shear (Strength) | | | | | Comments: |
|------------------|--------------------|-------------------|---------|---------------|--------------|------|------|---------------------------------------|---------------|--------|----------|----------------|--------------------------|---------------|--------|----------|----------------|---------------|
| | | | | | LL | DC | DW | Distribution Factor (DF) | Rating Factor | Tons | Location | Dimension (Ft) | Distribution Factor (DF) | Rating Factor | Tons | Location | Dimension (Ft) | |
| 30 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.41 | 2.08 | N/A | C | 15.00 | 0.68 | 4.26 | N/A | A | 1.30 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.41 | 2.70 | N/A | C | 15.00 | 0.68 | 5.52 | N/A | A | 1.30 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.41 | 2.48 | N/A | C | 15.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.41 | 2.27 | 136.41 | B | 12.24 | 0.68 | 3.76 | 225.81 | A | 1.30 | Exterior Beam |
| 40 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.38 | 1.35 | N/A | C | 20.00 | 0.66 | 4.48 | N/A | A | 4.00 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.38 | 1.74 | N/A | C | 20.00 | 0.66 | 5.81 | N/A | A | 4.00 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.38 | 1.75 | N/A | C | 20.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.38 | 1.33 | 79.92 | C | 20.00 | 0.66 | 4.03 | 241.95 | A | 4.00 | Exterior Beam |
| 50 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.37 | 1.25 | N/A | C | 25.00 | 0.65 | 4.08 | N/A | A | 1.50 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.37 | 1.61 | N/A | C | 25.00 | 0.65 | 5.28 | N/A | A | 1.50 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.37 | 1.66 | N/A | C | 25.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.37 | 1.20 | 71.84 | C | 25.00 | 0.65 | 3.78 | 227.02 | A | 1.50 | Exterior Beam |
| 60 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.36 | 1.50 | N/A | C | 30.00 | 0.64 | 5.02 | N/A | A | 6.00 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.36 | 1.94 | N/A | C | 30.00 | 0.64 | 6.50 | N/A | A | 6.00 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.36 | 1.58 | N/A | C | 30.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.36 | 1.47 | 88.16 | C | 30.00 | 0.64 | 4.73 | 283.54 | A | 6.00 | Exterior Beam |

- NOTES:
1. Permit capacity is determined by using the permit vehicle in all lanes.
 2. Service III Design Inventory tensile stress limits:
 $3\sqrt{f'_c}$ (Extremely/Moderately Aggressive)
 $6\sqrt{f'_c}$ (Slightly Aggressive)
 3. Meets AASHTO LRFD Specifications Article 5.8.3.5 longitudinal reinforcement criteria.

Abbreviations:

Inv - Inventory

Op - Operating



RATING LOCATIONS



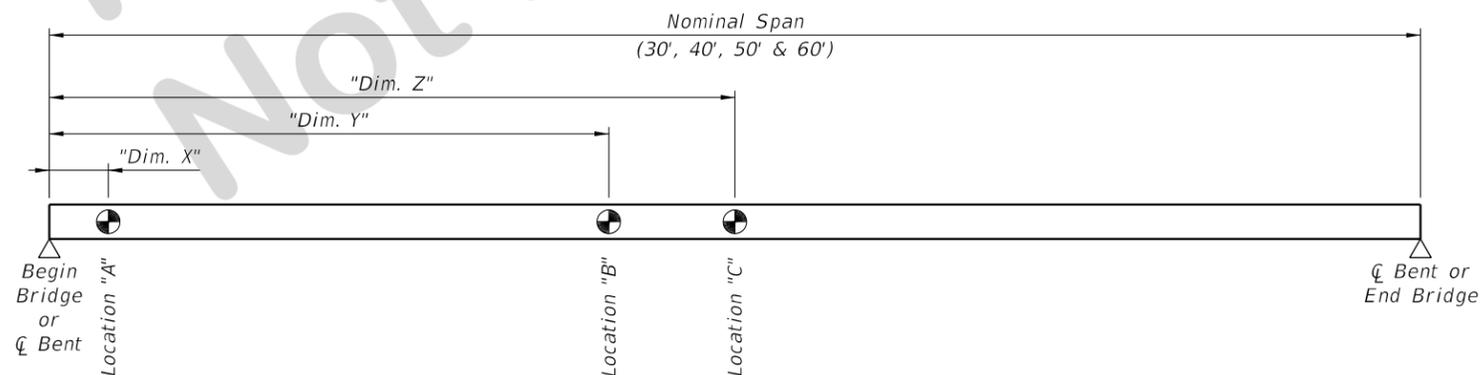
Table 2 - LRFR - Load Rating Summary Details for Prestressed Concrete Bridges (Flat Slab and Deck/Girder)

Superstructure - Environmental Classification: Extremely Aggressive

| Span Length (Ft) | Level | Limit State | Vehicle | Weight (tons) | Load Factors | | | Moment (Strength) or Stress (Service) | | | | | Shear (Strength) | | | | | Comments: |
|------------------|--------------------|-------------------|---------|---------------|--------------|------|------|---------------------------------------|---------------|--------|----------|----------------|--------------------------|---------------|--------|----------|----------------|---------------|
| | | | | | LL | DC | DW | Distribution Factor (DF) | Rating Factor | Tons | Location | Dimension (Ft) | Distribution Factor (DF) | Rating Factor | Tons | Location | Dimension (Ft) | |
| 30 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.41 | 2.08 | N/A | C | 15.00 | 0.68 | 4.26 | N/A | A | 1.30 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.41 | 2.70 | N/A | C | 15.00 | 0.68 | 5.52 | N/A | A | 1.30 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.41 | 2.04 | N/A | C | 15.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.41 | 2.27 | 136.41 | B | 12.24 | 0.68 | 3.76 | 225.81 | A | 1.30 | Exterior Beam |
| 40 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.38 | 1.35 | N/A | C | 20.00 | 0.66 | 4.48 | N/A | A | 4.00 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.38 | 1.74 | N/A | C | 20.00 | 0.66 | 5.81 | N/A | A | 4.00 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.38 | 1.43 | N/A | C | 20.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.38 | 1.33 | 79.92 | C | 20.00 | 0.66 | 4.03 | 241.95 | A | 4.00 | Exterior Beam |
| 50 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.37 | 1.25 | N/A | C | 25.00 | 0.65 | 4.08 | N/A | A | 1.50 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.37 | 1.61 | N/A | C | 25.00 | 0.65 | 5.28 | N/A | A | 1.50 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.37 | 1.34 | N/A | C | 25.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.37 | 1.20 | 71.84 | C | 25.00 | 0.65 | 3.78 | 227.02 | A | 1.50 | Exterior Beam |
| 60 | Design Load Rating | Strength I (Inv) | HL-93 | N/A | 1.75 | 1.25 | 1.50 | 0.36 | 1.50 | N/A | C | 30.00 | 0.64 | 5.02 | N/A | A | 6.00 | Exterior Beam |
| | | Strength I (Op) | HL-93 | N/A | 1.35 | 1.25 | 1.50 | 0.36 | 1.94 | N/A | C | 30.00 | 0.64 | 6.50 | N/A | A | 6.00 | Exterior Beam |
| | | Service III (Inv) | HL-93 | N/A | 0.80 | 1.00 | 1.00 | 0.36 | 1.26 | N/A | C | 30.00 | N/A | N/A | N/A | N/A | N/A | Exterior Beam |
| | Permit Load Rating | Strength II | FL120 | 60.0 | 1.35 | 1.25 | 1.50 | 0.36 | 1.47 | 88.16 | C | 30.00 | 0.64 | 4.73 | 283.54 | A | 6.00 | Exterior Beam |

- NOTES:
1. Permit capacity is determined by using the permit vehicle in all lanes.
 2. Service III Design Inventory tensile stress limits:
 $3\sqrt{f'_c}$ (Extremely/Moderately Aggressive)
 $6\sqrt{f'_c}$ (Slightly Aggressive)
 3. Meets AASHTO LRFD Specifications Article 5.8.3.5 longitudinal reinforcement criteria.

Abbreviations:
 Inv - Inventory
 Op - Operating



RATING LOCATIONS

