

NOTES:

- 1. A bypass switch shall be installed to override each timing interval in case of a malfunction.
- 2. "STOP HERE ON RED" is omitted in Type I operation and "TRAFFIC SIGNALS" are omitted in Type II operation.
- 3. The time between beginning of flashing yellow on "Drawbridge Ahead" sign and the clearance of traffic signal to red, or beginning of flashing red should not be less than the travel time of a passenger car, from the sign location to the stop line, traveling at the 85 percentile approach speed.
- 4. Beginning of operation of drawbridge gates shall not be less than 15 seconds after steady red or 20 seconds after flashing red (Actual time may be determined by the bridge tender.)
- 5. Time of gate lowering and raising is dependent upon gate type.
- 6. Time of bridge opening is determined by the bridge tender.
- 7. Each gate shall be operated by a separate switch.
- 8. On each approach (Type II), all four red signals shall be on the same two circuit flashers, with the two top signals on one circuit, and the two bottom signals on the alternately flashing circuit.
- 9. A Drawbridge Ahead sign is required for both types of signal operation, However a flashing beacon shall be added to the sign when physical conditions prevent a driver traveling at the 85% approach speed from having continuous view of at least one signal indication for approximately 10 seconds.
- 10. Requirements on gate installation are contained in the "Manual on Uniform Traffic Control Devices" Section 4J.
- 11. "In accordance with Traffic Engineering Manual (Topic Number 750-000-005) Section 2.1, SLIPPERY WHEN WET SIGNS shall be placed in advance of all MOVABLE and NONMOVABLE STEEL DECK BRIDGES."

LEGEND:

- ① TRAFFIC SIGNALS] Mast Arm Mounted (Off Bridge)
- ② DRAWBRIDGE SIGN Monotube Support Mounted (On Bridge)
- ③ DRAWBRIDGE AHEAD SIGN WITH YELLOW FLASHING BEACON
- ④ STOP HERE ON RED SIGN WITH RED FLASHING BEACONS
- 5 ENTRANCE GATE
- 6 EXIT GATE
- ① 24" THERMOPLASTIC STOP BAR

(- SIGNAL SWITCH	OFF			
	FLASHING BEACON DRAWBRIDGE AHEAD SIGN (See Note 9)	BLANK			
STONALS & STONS	STOP HERE ON RED (Type II only)	BLANK	<		
	TRAFFIC SIGNALS (Type I only)	GREE	Ν	YELLOV	V ::::::::::
ſ	ENTRANCE GATES		RAISED		
GATES <	EXIT GATES				
			Variable Time	5 Sec.	15 Sec.
	TIMING		(See Note 3)	Min.	Variable (See Note
		Normal			Opera
		Operation	-		
			== SEQU	IENC	E CHAR



COLORIZATION:



Ground Mounted

FY 2024-25 STANDARD PLANS TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS





REVISION



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MEDIAN SECTION AT SIGNAL GATES

= MEDIAN SIGNAL GATES FOR MULTILANE UNDIVIDED URBAN SECTIONS $=\!=\!$ (Three or More Driving Lanes in one Direction, 45 mph or less)

LAST REVISION 11/01/23

DESCRIPTION:



FY 2024-25 STANDARD PLANS

RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES







3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS				
MEMBER	DESIGNATION	OUT SIDE DIMENSION	WALL THICKNESS	
Post "A"	HSS $2\frac{1}{2} \times 1\frac{1}{2} \times \frac{1}{8}$	2.50" x 1.50"	0.125"	
Post "B"	HSS $2\frac{1}{2} \times 1\frac{1}{2} \times \frac{3}{16}$	2.50" x 1.50"	0.188"	
Top Bail	2½" NPS (Sch. 10)	2.875"	0.120"	
	HSS 3.000 x 0.120	3.000"	0.120"	
End Hoops	2½" NPS (Sch. 10)	2.875"	0.120"	
	HSS 3.000 x 0.120	3.000"	0.120"	
Top Rail Joint/Splice Sleeves	HSS 2.500 x 0.125	2.500"	0.125"	
Intermediate & Bottom Rail	HSS 2 x 2 x $\frac{3}{16}$	2.00" x 2.00"	0.188" ⁽¹⁾	
Int. & Bottom Rail Post Connection Sleeve	HSS 1.500 x 0.125	1.500"	0.125" ⁽¹⁾	
Handrail Joint/Enlice Cleaves	1" NPS (Sch. 40)	1.315"	0.133"	
Tanuran Jonit/Sprice Sleeves	HSS 1.500 x 0.125	1.500"	0.125"	
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrail Support Bar	¾" Ø Round Bar	0.750"	N/A	
Pickets (Type 1 Infill Panel)	³₄" Ø Round Bar	0.750"	N/A	
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies	

TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be $1\frac{1}{4}$ " NPS (Sch. 40).

Notes:

1. Shop Drawings are required; see Specification Section 515

- 2. For bridge mounted railings work this Index with Index 515-051 Bridge Bicycle/Pedestrian Railing 3. Materials:
 - A. Pipe Rails and Pickets: ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) and ASTM A36 for bars.
 - B. Structural Tube: ASTM A500 Grade A, B, C, or D or ASTM A501
 - C. Steel Plate: ASTM A36 or ASTM A709 Grade 36
 - D. U-Channels and filler plates: ASTM A36 or ASTM A1011 (Grade 36).
 - E. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
 - F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962. a. Hex Head Bolts: ASTM A 307
 - 1. $\frac{7}{8}$ " diameter single bolt option, Grade 36
 - 2. $\frac{7}{16}$ " four bolt option, Grade 55
 - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
 - c. Hex Nuts: ASTM A563
 - d. Flat Washers: ASTM F436
 - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
 - G. Shims: ASTM B209 Alloy 6061
 - H. Bearing Pads: $\frac{1}{2}$ " Plain, Fabric Reinforced or Fabric Laminated pads that meet the requirements of Specification Section 932 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5%" for standard installations and 3%" when a 4" sphere requirement is indicated in the Data Tables.
- 5. Maximum spacing between expansion joints is 40'-0". Locate an Expansion Joint between the posts on either side of the Deck Expansion Joint.
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K". 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for: A. Grades Steeper than 5%,
 - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.





NOTES I

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DESCRIPTION: LAST







PEDESTRIAN/BICYCLE RAILIN

СНА	IN-LINK	PANEL COMPONENT MATERIALS
	ASTM	COMPONENT INFORMATION
h	A 392	Zinc-Coated Steel – No. 9 gage (coated wire diameter), Class 2 Coating
	A 491	Aluminum-Coated Steel – No. 9 gage (coated wire diameter)
	F 668	Polyvinyl Chloride (PVC) Coated Steel – No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.
	F 626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.
	F 626	³ ⁄ ₁₆ " (Min. thickness) x ³ ⁄ ₄ " (Min. width) x 2'-3' (Min. height) Steel Bars
е	F 626	Zinc-Coated Steel

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TYPICAL SECTION FOR 4~BOLT ANCHORAGE

CHOR BOLT TABLE					
DIMENSIONS ANCHOR LENGTH					
t.	B Edge Dist.	C Embedment	C-I-P Hex Head Bolt	Adhesive Anchor	SIZE
	1'-2"	6"	7½"	8"	7∕8″Ø
	4"	9"	10 ¹ ⁄2"	11"	7∕8" Ø
	3½" @ top	9"	10½"	11"	7∕8"Ø
	4½"	9"	10 ¹ ⁄2"	11"	7∕8" Ø
	5"	5"	$6^{1\!/\!2''}$	7"	7⁄16″Ø

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3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS				
MEMBER	ALLOY ⁽¹⁾	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS
Posts (Type "A" & "B")	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250"
Posts (Type "C")	6061-T6	Extrusion 1½x2½x0.125	1.50" x 2.50"	0.125"
Top Plate (Type "C")	6061-T6	Extrusion (See Details)	2¾" x 7"	Varies
Top Pail	6061 76	2½" NPS (Sch. 10)	2.875"	0.120"
Тор кап	0001-10	3" Round Top Cap Rail	3.000"	0.125"
Frid Haaris		2½" NPS (Sch. 10)	2.875"	0.120"
Ena Hoops	6063-15	3.00 OD x 0.125 Wall	3.000"	0.125"
Top Dail Joint/Collice Cleaves		2.50 OD x 0.125 Wall	2.500"	0.125"
Top Rail Joint/Spice Sieeves	0003-13	Top Cap Rail Inner Sleeve	2.800"	0.090"
Intermediate & Bottom Rail	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250" ⁽²⁾
Int. & Bottom Rail Post Connection Sleeve	6063-T5	1.50 OD x 0.125 Wall ⁽³⁾	1.500"	0.125"
Handrail laint/Calico Clasues	6063-T5	1" NPS (Sch. 40)	1.315"	0.133"
Handran Johnspirce Steeves	6063-T5	1.50 OD x 0.125 Wall	1.500"	0.125"
Handrails	6061-T6	1½" NPS (Sch. 40)	1.900"	0.145"
Handrail Support Bar	6061-T6	¾" Ø Round Bar	0.750"	N/A
Pickets (Type 1 Infill Panel)	6061-T6	¾" Ø Round Bar	0.750"	N/A
Infill Panel Members (Types 2 - 5)	6063-T5	Varies (See Details)	Varies	Varies

1. Shop Drawings are required, see Specification Section 515. 2. For bridge mounted railings, work this Index with Index 515-061 Bridge Bicycle/Pedestrian Railing (Aluminum)

3. Materials: A. Structural Extrusions, Tube, Pipe and Bars: Table 1 and ASTM B221 or ASTM B429 a. Top, bottom and intermediate rail corner bends with maximum 4'-0" post spacing may be Alloy 6063-T6

- B. Base Plates and Rail Caps: ASTM B209 Alloy 6061-T6
- C. Perforated panels (Type 5) Alloy 3003-H14
- D. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
- E. Aluminum screws: Alloy 2024-T4 or 7075-T73
- F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962. a. Hex Head Bolts: ASTM A 307
 - 1. $\frac{7}{8}$ " diameter single bolt option, Grade 36
 - 2. $\frac{7}{16}$ " diameter four bolt option, Grade 55
 - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
 - c. Hex Nuts: ASTM A563
 - d. Flat Washers: ASTM F436
 - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
- G. Shims: ASTM B209 Alloy 6061 or 6063
- Specification Section 932 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5%" for standard installations and 3%" when a 4" sphere requirement is indicated in the Data Tables.
- 5. Locate railing expansion Joints between the posts on either side of the deck expansion joint. Maximum spacing between expansion joints is 35'-0".
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown
- in detail "K" for Post Type "A" & "B". 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for: A. Grades Steeper than 5%,

Detail "A", Sheet 4

Detail "B", Sheet 4

Detail "K", Sheet 3

B. Three or more steps

CROSS REFERENCES:

12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

TABLE 1 NOTES:

- (1) Alloy 6061-T6 or 6063-T52 & T6 may be substituted for Alloy 6063-T5.
- (2) 0.188" wall thickness permitted for rails with post spacings less than 5'-9".
- (3) 1" NPS (Sch. 40) non-slit rail sleeves may be substituted when welded

connection Detail "K" is utilized.



 \geq DESCRIPTION:

LAST

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3" ROUND TOP CAP RAIL TOP CAP RAIL INNER SPLICE SLEEVE

ALTERNATE TOP RAIL SECTION

2"

ALTERNATIVE BOTTOM & INTERMEDIATE RAIL SECTION FOR TYPE 3, 4 & 5 RAILINGS

See Screw Slot Detail (Тур.) $1\frac{1}{2}''$

POST TYPE "C" SCREW SLOT SECTION



SCREW SLOT DETAIL

PEDESTRIAN/BICYCLE RAILING

FY 2024-25

FDOŤ STANDARD PLANS

= NOTES =

H. Bearing Pads: Provide 1/2" thick Plain, Fabric Reinforced or Fabric Laminated Bearing Pads meeting the requirements of



OPTIONAL TOP PLATE EXTRUSION SECTION (POST TYPE "C")

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СНА	IN-LINK	PANEL COMPONENT MATERIALS
-	ASTM	COMPONENT INFORMATION
h	A392	Zinc-Coated Steel – No. 9 gage (coated wire diameter), Class 2 Coating
, ,	A491	Aluminum-Coated Steel – No. 9 gage (coated wire diameter)
	F668	Polyvinyl Chloride (PVC) Coated Steel – No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.
	F626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.
	F626	³⁄ ₁₆ " (min. thickness) x ³⁄₄" (min. width) x 2'-3' (min. height) Steel Bars
е	F626	Zinc-Coated Steel

JUMINUM)	

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TYPICAL SECTION FOR 4~BOLT ANCHORAGE

CHOR BOLT TABLE						
DIMENSIONS ANCHOR LENGTH						
t.	"B" Edge Dist.	"C" Embedment	C.I.P Hex Head Bolt	Adhesive SIZE Anchor		
	1'-2"	6"	7½"	8"	7∕8″Ø	
	4"	9"	10½"	11"	7∕8" Ø	
	3½" @ top	9"	10½"	11"	7∕8"Ø	
	4½"	9"	10½"	11"	7∕8" Ø	
	5"	5"	6½"	7"	7 <u>/16</u> " Ø	

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NOTES:

- 1. Shop Drawings are required.
- 2. Work with Specification 515.
- 3. Materials:
- A. Pan Head Set Screws: Aluminum Alloy 2024-74 or 7075-T73 or Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM B209, Alloy 6061-T6
- C. Structural Pipe Tube and Bars: ASTM B221 or ASTM B429, Alloy 6061-T6

D. End Rails 90° bends and corner bends with a maximum 4 foot spacing: Alloy 6063-T5 is permitted.

RAILING MEMBER DIMENSIONS TABLE					
MEMBER	DESIGNATION	OUT SIDE DIMENSION	WALL THICKNESS		
Posts	2" NPS (Sch. 40)	2.375"	0.154"		
Rails	2" NPS (Sch. 40)	2.375"	0.154"		
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) 1.50 0Dx0.125 Wall	1.315" 1.500"	0.133" 0.125"		
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A		

E. Galvanized Steel Fasteners:

- a. Hex Head Bolts: ASTM A 307 Type 1 or ASTM F1554 Grade 36
- b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- F. Aluminum Shims: ASTM B209, Alloy 6061

G. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specifications 515 & 932.

4. Fabrication:

- A. Place expansion joints at a maximum of 30'-0"spacing
 B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts.
 C. Continuity field splice (Detail "E"); only use to make the railing continuous for unforeseen field adjustments
 D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9" bend radius or terminated at adjoining sections with a standard end hoop when handrails are not required.
- E. For curved longitudinal alignments, shop bend top and bottom rails and handrails to match the alignment radius. F. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner, not at the corner apex.

5. Handrails are required and must be continuous at landings for:

- A. Grades Steeper than 5%
- B. Three or more steps

6. Cutting of reinforcing steel is permitted for post installed anchor bolts.





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NOTES:

1. Shop Drawings are required, refer to Specification 515.

2. <u>Materials:</u>

- A. Pan Head Set Screws: Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM A36 or ASTM A709 Grade 36
- C. Pipe Rails and Posts: ASTM A53 Grade B for standard weight pipe and ASTM A500 Grade B, C or

D or ASTM A501 for Structural Tube.

Handrail Support Bars: ASTM A36

RAILING MEMBER DIMENSIONS TABLE					
MEMBER	DESIGNATION	OUT SIDE DIMENSION	WALL THICKNESS		
Posts	2" NPS (Sch. 40)	2.375"	0.154"		
Rails	2" NPS (Sch. 40)	2.375"	0.154"		
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) HSS1.500x0.125	1.315" 1.500"	0.133" 0.125"		
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A		

D. Galvanized Steel Fasteners:

a. Hex Head Bolts: ASTM A307 Type 1 or ASTM F1554 Grade 36

b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods

- c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- E. Aluminum Shims: ASTM B209, Alloy 6061
- F. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specifications 515 and 932.

3. Fabrication:

- A. Place expansion joints at a maximum of 30'-0"spacing.
- B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts.
 C. Continuity field splice (Detail "E") only use to make the railing continuous for unforeseen field adjustments
- D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9"bend radius or terminated
- at adjoining sections with a standard end hoop when handrails are not required. E. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- F. For changes in tangential longitudinal alignment greater than 45°, positioned posts a maximum of 2'-0" each side of the corner, not at the corner apex.

4. Handrails are required and must be continuous at landings for:

- A. Grades Steeper than 5%,
- B. Three or more steps
- 5. Cutting of reinforcing steel is permitted for adhesive anchor bolt installations.





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EEL)	INDEX	SHEET
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GENERAL NOTES:

- 1. For curb, gutter and curb & gutter provide $\frac{1}{8}$ " $\frac{1}{4}$ " contraction joints at 10' centers (max.). Contraction joints adjacent to concrete pavement on tangents and flat curves are to match the pavement joints, with intermediate joints not to exceed 10' centers.
- 2. Locate expansion joints for curb, gutter and curb & gutter in accordance with Specification 520.

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1	General Notes and Contents	
2	Concrete Curb and Gutter	
3	Curb and Gutter Joints and Endings, Concrete Bumper Guard, and Asphaltic Concrete Curb	







TYPE F



TYPE A, TYPE E, TYPE F, AND SHOULDER GUTTER (Other Types Similar)



— CONCRETE BUMPER GUARD ——





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SHOULDER GUTTER



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LAST

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ROADWAY INSTALLATIONS - RIGID PAVEMENT

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DGE INSTALLATIONS	- TYPE	"E" CURB
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NOTES:

- 1. Treatment of separators on straight bridges shown. For additional notes and treatment of separators on skewed bridges, see Sheet 2.
- 2. Option II is not permitted on bridge decks with prestressing steel.
- 3. Bar Spacing: 4'-0" @ 3 equal spaces (continuous) 6'-0" @ 5 equal spaces (continuous) 8'-6" @ 7 equal spaces (continuous)
- 4. At the Contractor's option, a one piece bar my be substituted for Bars 4B and 4E.
- 5. Field bend and cut rebar as required to maintain cover.

DGE INSTALLATIONS	- TYPE	"F" CURB
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ESTIMATED TRAFFIC SEPARATOR QUANTITIES:

CONCRETE: CONSTANT WIDTH OF SEPARATOR: <u>TYPE "E"</u> <u>TYPE "F"</u> 4'-0" Width = 0.056 CY per Ft. - 0.072 CY per Ft. 6'-0" Width = 0.089 CY per Ft. - 0.112 CY per Ft. 8'-6" Width = 0.132 CY per Ft. - 0.164 CY per Ft. NOSE: <u>TYPE "E"</u> 4'-0'' Width = 0.080 CY - 0.109 CY 6'-0'' Width = 0.193 CY - 0.257 CY 8'-6'' Width = 0.403 CY - 0.536 CY REINFORCING STEEL: (All quantities are based on an $8^{1}/_{3}$ " slab.) OPTION I: 4'-0" Width - 6.37 Lbs. per Ft. 6'-0" Width - 8.60 Lbs. per Ft. 8'-6" Width - 11.05 Lbs. per Ft. OPTION II: 4'-0" Width - 4.77 Lbs. per Ft. 6'-0" Width - 7.00 Lbs. per Ft. 8'-6" Width - 9.45 Lbs. per Ft.

BRIDGE INSTALLATIONS - TYPE "E" AND "F" CURB

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3	Median Barrier – Reinforcing Details
4	Median Barrier – Sloped End Treatment
5	Median Barrier – Grade Separated
6	Median Barrier – 56" Height Section for Barrier-Mounted Sign Support Shielding – Symmetrical
7	Median Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding - Asymmetrical
8	Median Barrier - 56" Height Section for Barrier-Mounted Dual Sign Support Shielding - Min. Width
9	Median Barrier - 38" Height Split Section for Stand-Alone Sign Support Shielding
10	Median Barrier – 44" Height Split Section for Pier Shielding
11	Median Barrier – 44" Height Split Section for Pier Shielding – Details
12	Median Barrier – Connection to F-Shape
13	Shoulder Barrier
14	Shoulder Barrier – Reinforcing Details
15	Shoulder Barrier – Section Options
16	Shoulder Barrier – Section Options (Continued), Drainage Slot Option
17	Shoulder Barrier - 38" Height Rear-Flush Section for Reduced Setback Pier Shielding (Low-Speed)
18	Shoulder Barrier – 44" Height Rear-Flush Section for Reduced Setback Pier Shielding
19	Shoulder Barrier – Connection to F-Shape
20	Curb and Gutter Barrier
21	Curb and Gutter Barrier – Reinforcing Details
22	Curb and Gutter Barrier – Sloped End Treatment
23	Wall Shielding Barrier - 38" Height Section - Approach and Trailing Transition
24	Wall Shielding Barrier - 38" Height Section - Guardrail Connection
25	Wall Shielding Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding
26	Reinforcing Bar Bending Diagrams

GENERAL NOTES:

- 1. BARRIER CONCRETE: Use Class II concrete for all barriers constructed in slightly aggressive environments, and use Class IV Concrete for all barriers constructed in moderately or extremely aggressive environments. On all exposed surfaces, apply a General Surface Finish in accordance with Specification 400.
- 2. STEEL BAR REINFORCEMENT: Where required to maintain continuity, provide lap splices of at least 18 inches for No. 4 bars and 20 inches for No. 5 bars, unless otherwise shown herein (including shorter splices as provided by the default bar bending diagrams).

The default reinforcing details shown herein, including bar shapes and lap splice positions, are intended to show required steel locations and provide for a constructible design. However, with the approval of the Engineer, alternate steel configurations may be used in the same locations shown herein, given that the equivalent strength reinforcing is provided and the cover, maximum spacing, and continuity requirements are maintained.

3. OPTIONAL WELDED WIRE REINFORCEMENT: With the approval of the Engineer, steel welded wire reinforcement in accordance with Specification 415 may be substituted for the steel bars shown herein. Place the welded wire in the same locations specified for the steel bars, and maintain the equivalent strength, cover, maximum spacing, and continuity requirements.

GENERAL NOTES (CONTINUED):

- 4. TOP FACE LONGITUDINAL REINFORCEMENT: Unless otherwise specified, the longitudinal reinforcement shown closest to the top face of the barrier has a maximum cover of $4^{1}/_{2}$ ", measured from the top face of the barrier.
- 5. MINIMUM BARRIER LENGTH: Unless otherwise shown in the Plans, the minimum Concrete Barrier length is 40 feet.
- 6. CONSTRUCTION JOINTS: Install Construction Joints only as needed for discontinuous concrete casting or cold joints. Maintain continuity of steel reinforcement across Construction Joints. Construction Joints are classified herein as Transverse Joints or Longitudinal Joints.

Transverse Joints are permitted at 20-foot or greater intervals along the barrier. For Tall Grade-Separated Sections, see Sheet 5 for additional Transverse Joint requirements.

Longitudinal Joints are only permitted where indicated in the following details and notes, with a vertical position tolerance of $\pm 1^{1/2''}$ from the locations shown.

- 7. DOWELED JOINTS: As shown in the Dowel Details on Sheets 2 & 13, install $\frac{3}{4}$ " Doweled Joints for Concrete Barrier connections to Wall Coping Barriers, Pier Protection Barriers, and Traffic Railings. Doweled Joints are also required for expansion mitigation in Median Barrier as defined per Sheets 2 & 5. Doweled Joints are not permitted within Grade-Separated Median Barrier.
- 8. CRACK CONTROL V-GROOVES: At 20-foot intervals, place $\frac{3}{6}$ " depth V-grooves that run vertically and/or transversely in the front, top, and back faces of barriers. The V-grooves can be either molded or scored while the concrete is still plastic.
- 9. SUBGRADE: Compact the top layer of subgrade with Type B Stabilization, LBR 40 (12 in.).
- 10. FOOTING BOTTOM CONCRETE COVER: At the bottom of barrier footings shown throughout this Index, up to 2 inches of additional concrete cover is permitted beyond what is shown herein to accommodate soil grade irregularities.
- 11. FINISH GRADE ELEVATION: At the barrier face location, the finish grade pavement has a vertical position tolerance of $\pm \frac{1}{2}$ " from the nominal locations shown herein, relative to the barrier elevation. Maintain visually smooth and even pavement at the barrier face, per the approval of the Engineer.
- 12. DRAINAGE INLETS: Where called for in the Plans, install corresponding inlets per Indexes 425-030 thru 425-032.
- 13. LIGHT POLE MOUNTING: Where called for in the Plans, install aluminum light poles per Index 715-002.
- 14. OPAQUE VISUAL BARRIER: Where called for in the Plans, install Opaque Visual Barrier per Index 521-010.
- 15. BARRIER END MARKERS: For all free ends of concrete barriers that are not shielded with an end treatment or connection to another barrier or traffic railing type, install a Type 3 Object Marker on the end face per Specification 705.
- 16. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification 705. For median barriers, mount the delineator on the top of the barrier, at the centerline of barrier, with reflective sheeting facing traffic on both approaches. For shoulder barriers and split sections, mount the delineators on the top of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.
- 17. TOLL SITES: Where called for in the Plans, substitute the steel reinforcing bars shown herein with GFRP reinforcing bars of the same size. Construct GFRP reinforcing bars in accordance with Specification 932, and use a maximum 4½" inner diameter for bar bends. Alternative bar bending details and shapes may be used so long as the final location of the reinforcing is unchanged and the bars are either continuous or fully spliced at the side and bottom barrier locations. Where required to fit pull boxes while maintaining bar spacing and concrete cover, trim GFRP bars as defined in the Plans.

At toll site locations, the use of Median Barriers on outside shoulders is permitted where called for in the Plans. Shoulder Pavement shown herein may be substituted with material for an alternate usage where defined in the Plans.

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MEDIAN BARRIER

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PLAN VIEW - 38" HEIGHT MEDIAN BARRIER FREE END REINFORCING (See Note 3)



PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (See Note 3)



38" HEIGHT MEDIAN BARRIER Concrete Qty. = 0.20 CY/FT Steel Qty. = 11.8 LB/FT

DESCRIPTION:



FY 2024-25 STANDARD PLANS

CONCRETE BARRIER

NOTES:

- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 2.
- 2. BAR BENDING DIAGRAMS: For additional information on Bars 4V1 and 4U1, see the details on Sheet 26.
- 3. PLAN VIEWS: Only top and bottom longitudinal reinforcing is shown for clarity. For all longitudinal steel locations, see the section views.







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Grade-Separated sections using a continuous pour or Transverse Joint, where longitudinal steel

that aligns within the adjacent section is maintained continuously between sections or has a full

lap splice with the adjacent section's longitudinal steel. Connect Short Grade-Separated sections

3. SHORT GRADE-SEPARATED SECTIONS: Bars 4C1 and the two uppermost longitudinal bars may be

4. TALL GRADE-SEPARATED SECTIONS: For the vertical and transverse steel reinforcement shown in the Tall Grade-Separated Sections, bar bending diagrams are not provided due to varying section dimensions and Longitudinal Joint locations. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown

Longitudinal Joints are permitted between the vertical limits shown, and must remain level and at a consistent height per each continuous casting of concrete. Longitudinal Joints may change

elevations at Transverse Joint locations. Field bending of bars is permitted at Longitudinal Joint

and 38" Height Median Barrier sections of Sheet 2 using a $\frac{3}{4}$ " Doweled Joint.



Shoulder



TALL GRADE-SEPARATED HEEL FOOTING SECTION FOR $Y \leq 4'-0''$

TALL GRADE-SEPARATED SECTIONS DIMENSION TABLE							
Max. Height, Y	1'-0"	1'-6"	2'-0"	2'-6"	3'-0"	3'-6"	4'-(
ooting Width, W	3'-3"	3'-6"	3'-9"	4'-0''	4'-3''	4'-6"	4'-6

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locations.

NOTES:

Grade-Separated sections.

omitted for segments where Y < 2''.

herein, as approved by the Engineer.

steel reinforcement.

sides of the joint.

DESCRIPTION:



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CONCRETE BARRIER

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_				

1. PROJECT-SPECIFIC REINFORCING: For footing and barrier reinforcing required for the overhead sign support, see the project-specific design in the Plans.

2. BARRIER REINFORCING: Maintain the 38" Height Median Barrier's longitudinal steel reinforcing continuously through the barrier height transition and pedestal. Provide the Additional No. 4 Bars and taper as required to maintain a

For the vertical and transverse reinforcement requirements shown in Sections A-A through C-C, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength

3. PROJECT-SPECIFIC PEDESTAL WIDTH & SETBACK: The pedestal width is governed by the size requirements of the overhead sign support, as detailed in the Plans. Likewise, the setback distance from the sign support base to the barrier face is governed by the anchor bolt cover requirements, as defined per the Plans. The minimum pedestal width is 2'-0", where a complete removal of the

4. PLAN VIEW: Only top and bottom longitudinal reinforcing is shown for clarity.

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1. PROJECT-SPECIFIC REINFORCING: For footing and barrier reinforcing required for the overhead sign support, see the project-specific design in the Plans.

2. BARRIER REINFORCING: Maintain the 38" Height Median Barrier's longitudinal steel reinforcing continuously through the barrier height transition and pedestal. Provide the Additional No. 4 Bars and taper as required to maintain a

For the vertical and transverse reinforcement requirements shown in Sections A-A through C-C, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength

3. PROJECT-SPECIFIC PEDESTAL WIDTH & SETBACK: The pedestal width is governed by the size requirements of the overhead sign support, as detailed in the Plans. Likewise, the setback distance from the sign support base to the barrier face is governed by the anchor bolt cover requirements, as defined per the Plans. The minimum pedestal width is 2'-0", where a complete removal of the

4. PLAN VIEW: Only top and bottom longitudinal reinforcing is shown for clarity.

MEDIAN BARRIER - 56" HEIGHT SECTION FOR BARRIER-MOUNTED SIGN SUPPORT SHIELDING - ASYMMETRICAL

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STANDARD PLANS

CONCRETE BARRIER









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DER	BARRIER	-	REINF	ORCING	DETAILS

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RETAINING SECTION

Concrete Qty. = 0.36 CY/FT Steel Qty. = 55.3 LB/FT

NOTES:

- 1. GENERAL: Install the differing Section Options as required per the Plans.
- 2. CONNECTIONS BETWEEN DIFFERENT SECTIONS: Connect differing Shoulder Barrier sections using a continuous pour or Transverse Joint, where longitudinal steel that aligns within the adjacent section is maintained continuously between sections. Alternatively, a Doweled Joint may be used as shown on Sheet 13.
- 3. FLUSH RETAINING SECTION COMBINATION: Where Barrier Inlets are required in retaining segments, install the Flush Section, except replace the 1'-0" General Heel with the 2'-0" Extended Heel as shown in the Retaining Section. Use longer lateral reinforcing bars of 2'-10" length to maintain the cover shown.

Shoulder

Pavement ·



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CONCRETE BARRIER

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NOTES:

end face.



STANDARD PLANS

CURB AND GUTTER BARRIER -REINFORCING DETAILS

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1. PROJECT-SPECIFIC REINFORCING: For footing and barrier reinforcing required for the overhead sign support, see the project-specific design

2. BARRIER REINFORCING: Maintain the 38" Height Median Barrier's longitudinal steel reinforcing continuously through the barrier height transition and pedestal. Provide the Additional No. 4 Bars and taper as required to maintain a $4\frac{1}{2}$ " maximum cover from the top of the barrier.

For the vertical and transverse reinforcement requirements shown in Sections A-A through C-C, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the

3. PROJECT-SPECIFIC PEDESTAL WIDTH & SETBACK: The pedestal width is governed by the size requirements of the overhead sign support, as detailed in the Plans. Likewise, the setback distance from the sign support base to the barrier face is governed by the anchor bolt cover

4 TRANSITION SEGMENTS: The Transition Segments shown are examples only. For the actual approach and trailing transition taper rate, length,

5. PLAN VIEW: Only top and bottom longitudinal reinforcing is shown for clarity. For all longitudinal reinforcing locations, see the Section Views.

WALL SHIELDING BARRIER - 56" HEIGHT SECTION FOR BARRIER-MOUNTED SIGN SUPPORT SHIELDING

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BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
C1	4	3'-8"		
C2	5	3'-0"		
U1	4	5'-1"		
U2	4	7'-8"		
U3	5	9'-7"		
U4	5	5'-9"		
V1	4	6'-4"		
V2	5	6'-3"		
V3	4	5'-10"		

1. Work with the Standard Bar Bending Details per Index 415-001.

2. All bar dimensions in the bending diagrams are out to out.

3. Use standard inner diameters for bar bending unless otherwise shown.







BAR 4U2









BAR 5U3



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NOTES:





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CONCRETE BARRIER









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1	Index Contents; General Notes
2	Example Layouts - Footing Placement and Connections
3	Barrier Plan and Elevation - Connection to Concrete Barrier - Connection to Guardrail
4	Barrier Details - Connection to Concrete Barrier
5	Barrier Details - Connection to Guardrail
6	Barrier Footing Options
7	Crash Wall Details
8	Reinforcing Bar Bending Diagrams

GENERAL NOTES:

1. CONCRETE: Use Class III or IV concrete unless otherwise called for in the Plans.

2. CONSTRUCTION JOINTS: Maintain continuity of reinforcement steel across Construction Joints; reinforcement lap splices are peri immediately adjacent to joints. Construct all Pier Protection Barrier continuously, with no expansion or contraction joints. Const Joints are classified herein as Transverse Joints or Longitudinal Joints.

Transverse Joints are permitted at 40 foot or greater intervals along the barrier.

Longitudinal Joints may only be installed where indicated in the following details and notes, with a location tolerance of ± 1 " from the locations shown.

- 3. FOUNDATION: Compact the top 12 inches of the subgrade to at least 98% of the maximum density determined by FM 1-T 180, Method D.
- 4. DRAINAGE INLETS: See Index 425-031 for Adjacent Barrier Inlets, and isolate these structures from Pier Protection Barriers and Footings with 1" Preformed Joint Filler.
- 5. BARRIER END MARKERS: For all free ends of barriers that are not connected to guardrail or concrete barrier, install a Type 3 Object Marker on the end face per Specification 705.
- 6. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification 705. Mount the delineators on the top face of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.
- 7. CRACK CONTROL: Provide ½" depth crack control V-Grooves at 15' to 30' spacing. Locate V-Grooves above any joint or discontinuity in the barrier footing. Align V-Grooves perpendicular to the longitudinal axis of the Pier Protection Barrier and make continuous across the top surface and both side faces. For slip formed barriers, score $\frac{1}{2}$ " V-Grooves while the concrete is still plastic, otherwise pre-form the joints when stationary forms are utilized.





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STANDARD PLANS

2'-8¾'' 44" or 56" PPB (See Sheets 3 thru 5) Expanded Shoulder (Varies, 6'-0" Min.) Bars C. 5F2 @ Š. 6" Sp. Bottom of Spread Footing C. 2 Sp. @ 1'-0"

SECTION F-F SYMMETRICAL FOOTING OPTION

Concrete Qty. = 0.34 CY/FT (Below Gutter Line) Steel Qty. = 62.6 LB/FT (Including Bars 50)

1. GENERAL: Install the footing options per project-specific requirements, as defined on

Work with the supported 44" PPB and 56" PPB as shown on Sheets 3, 4, & 5.

3" laterally beyond the face of the stem to provide support for a subsequent slip forming operation above. Do not adjust the steel reinforcement location for the

3. GUARDRAIL CONNECTION TAPERED TOE: For tapering the barrier as shown on Sheet 5, View D-D, bend Bars U away from the stem face as required. For this case, the cover requirement is variable for one side of the stem (only at the tapered toe locations).

	BARRIER F	OOTING	OPTIONS
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BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
V	5	7'-5"		
U	5	8'-11"		
R	5	6'-0"		
F1	5	13'-9"		
F2	5	Varies (Straight)		
L	5	6'-5" / 7'-5"		
E	5	4'-6"		
<i>S1</i>	8	Varies (Straight)		
52, 53	5	Varies (Straight)		







NOTES:

- 1. Work with the Standard Bar Bending Details per Index 415-001.
- 2. All bar dimensions in the bending diagrams are out to out.



BARS 5F1

BARS 5U





ST OUSCRIPTION: SION US 1/17



PIER PROTECTION BAR



BARS 5R



BARS 5E

BAR BENDING DIAGRAMS

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GENERAL NOTES:

- 1. GENERAL: Construct Opaque Visual Barrier (OVB) in accordance with Specification 521. Use either cast-in-place or precast panels with Class II Concrete and Class 3 Surface Finish. Do not cast OVB concrete monolithically with the Concrete Barrier or Traffic Railing; use an ASTM D6380, Class S, Type III Organic Felt bond breaker as needed.
- 2. DOWEL BAR CONNECTION: For the embedment in Concrete Barrier or Traffic Railing concrete, dowel bars must be either cast in place for new concrete or grouted in place for existing concrete. Embed the dowel bars to the corresponding depths shown, and use the bar lengths provided in the Dowel Bar Length Table.

At cast in place embedment locations, longitudinally shift the dowel bars only as required to avoid reinforcing steel in the Concrete Barrier or Traffic Railing.

At grouted embedment locations, drill $\frac{5}{8}$ " Ø holes to a depth of $6\frac{1}{4}$ ". Use only approved non-shrink grout on the APL. Drilling through existing reinforcing steel is permitted.

3. TRANSVERSE JOINTS: Place $\frac{1}{2}$ " Transverse Joints with a maximum spacing of 50'-0" and a minimum spacing of 20'-0". Use a consistent spacing where practical.

Without violating the above spacing requirements, place Transverse Joints matching the location and width of open joints in the supporting Concrete Barrier or Traffic Railing.

- 4. SLOPED END TREATMENTS: Regardless of the traffic direction, place Sloped End Treatments on all exposed ends of OVB, excluding leave-outs for barrier-mounted signs and light poles. See Note 7 below.
- 5. BARRIER-MOUNTED SIGNS AND LIGHT POLES: Where signs and barrier-mounted light pole structures conflict with placement of OVB, end and restart the OVB with a transverse vertical face located a longitudinal distance of 2" ($\pm \frac{1}{2}$ ") from the base of the structure. Follow the same reinforcing scheme and concrete cover requirement for the Transverse Joint shown herein. See Note 7 below.
- 6. LARGE BARRIER-MOUNTED SIGN SUPPORTS: See Sheet 2 for details. See Note 7 below
- 7. LEAVE-OUTS: OVB leave-outs are longitudinal gaps in OVB segments required to accommodate barrier-mounted signs and light pole placement. Leave-outs up to 15 feet in length are included in OVB length measurement.
- 8. ASYMMETRICAL CONCRETE BARRIER SECTIONS: When mounting on top of an asymmetrical Concrete Barrier section (not shown), align the centerline of the OVB with the centerline of the top face of the Concrete Barrier section.
- 9. SPLIT CONCRETE BARRIER SECTIONS: For split Concrete Barrier sections that run separately (for vertical structures, bridges, etc.), OVB is only required on top of one of the Concrete Barrier sections. Place OVB on Pavement top of the Concrete Barrier section with the highest elevation. For these segments, mount barrier delineators on only the OVB face nearest the roadway (barrier mount other side). Longitudinally overlapping OVB runs are permitted where called for in the Plans, as designated with overlapping Begin and End Station OVB callouts.
- 10. VERTICAL REINFORCING: Place vertical No. 3 bars with the spacing shown, except that No. 3 bars at the dowel bar locations may be shifted longitudinally to fit or they may be omitted at the contractor's option.
- 11. OPTIONAL WELDED WIRE REINFORCEMENT: With the approval of the Engineer, the No. 3 bars shown herein may be replaced with welded wire reinforcement in accordance with Specification 415. Use welded wire reinforcement of equal or greater strength than the bars being replaced; maintain the same cover requirements with equivalent or smaller spacing.
- 12. VARIABLE HEIGHT CONCRETE BARRIERS: See Sheet 2 for details.
- 13. CONCRETE BARRIER AND TRAFFIC RAILING TRANSITIONS BETWEEN DIFFERING SECTIONS: Transition the OVB section using a method similar to the OVB Linear Bottom Transition shown in Elevation View 'B' on Sheet 2, except adjust the longitudinal length of the transition as required.

ELEVATION VIEW - OPAQUE VISUAL BARRIER



SECTION VIEW -**OPAQUE VISUAL BARRIER** FOR MEDIAN SINGLE-SLOPE CONCRETE BARRIER OR TRAFFIC RAILING

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DESCRIPTION: REVISION



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OPAQUE VISUAL BARRIER





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BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
R1	5	5'-2''		
R2	5	5'-2½"		
R3	5	4'-10''		
51	5	As Reqd.		
52	5	7'-3''		
V (Wall)	5	6'-6½"		
V (T-Footing)	5	7'-8½"		



REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.
- shall be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR
- 6. See Index 521-514 and 521-515 for L-shaped and Trench footing vertical reinforcing.

ESTIMATED TRAFFIC				
RAILING/NOISE WA	ALL QUANT	ITIES		
17 - 14				

ITEM	UNIT	QUANTITY
Concrete (Railing)	CY/LF	0.107
Concrete (Noise Wall)	CY/LF	0.136
Reinforcing Steel (Typical)	LB/LF	81.55
Additional Reinf. @ Open Joint	LB	241.58

(The above quantities are based on the Concrete Barrier/ Noise wall typical section, (excluding junction slab or footing)







CONCRETE BARRIER/NOISE WALL (8'-0")







oise Wall or End Taper (See Note 5)		" Open Joint
End 14'-0" Concrete Barrier/Noise	Wall (S	See Note 3)
Concrete Barrier		
	= = = = = = = !	= = = = = = = =
	1	Ţ
8'-0" Concrete Barrier/Noise Wal. End Taper on Approach Slab or F	continuing or Roadway (show	· · · · · · · · · · · · · · · · · · ·
		,
all and joints plumb; do not all perpendicular to the roadway surfac e with Specification Section 346. ssive environments.	ce.	
r extremely aggressive environments. Sendicular or radial to Gutter Line. Pro Sint locations are to coincide with $\frac{3}{4}$ " E de at 30'-0" maximum intervals as show Begin or End Concrete Barrier/Noise W	vide at 90'-0" xpansion Joints vn. Space V-Gro all.	in footings. ooves
when adjacent to an 8'-0" Concrete Ba rier/Noise Wall End Taper is provided e Barrier/Noise Wall End Treatment. oncrete Barrier/Noise Wall (8'-0") and	rrier/Noise Wa (see Index 521 one or	ll and -510 for
Noise Wall T-Shaped Spread Footing, Noise Wall L-Shaped Spread Footing of Noise Wall Trench Footing.	-	
		Begin or End
nin or End Concrete Barrier/Noise W nin or End 8'-0" End Taper (See Note	all 2 5 & 6)*	Railing/Noise Wall or End Taper (See Note 5 & 6)*
=========		
	======	
	l'	
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° · · · · · · · · · · · · · · · · · · ·		 Ţ
8'-0" Concrete Barrier/Noise Wall o End Taper on Approach Slab or Ro	continuing or adway (shown,)
Open Joint may be omitted when 8'-0 Per is adjacent to a 14'-0" Concrete	" Railing/Nois Barrier/Noise	e Wall End Wall End Taper
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Л	UNIT	QUANTITY	
v ₁		TYPE 1	TYPE 2
Slab)	CY/FT	0.268	0.305
Typical)	LB/FT	31.72	34.85
Expansion Joint	LB	21.36	21.36

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CONCRETE BARRIER/NOISE T-SHAPED SPREAD FOOTI

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LENGTH

AS REQD.



F	5	5'-6"
53	5	3'-7"
54	5	3'-10''
U 1	5	9'-2''
U2	5	13'-10''
U3	5	12'-10''
DOWEL	1" Ø Smooth Bar	2'-0''
4		

BILL OF REINFORCING STEEL

SIZE

5

В











BAR 5S4

REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-2".
- 4. Lap splices Bars 5T and 5V with 5U1 will be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.





BAR 5U3





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CONCRETE BARRIER/JUNCTION SLAB - WALL COPING

Bars 4P

Bars 5P

(*T* y p.)

Construction

@ 6" sp. (36")

@ 6" sp. (42")

– Bars 5L @ 1'–0" sp.

-3" Cover

Joint

Bars 4V1

Bars 5V1









TYPICAL SECTION THRU C-I-P CONCRETE BARRIER WITH C-I-P JUNCTION SLAB AND C-I-P COPING (PRECAST COPING SIMILAR WITH C-I-P BUILDUP)

NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. Vary the Junction Slab slope based on the roadway cross slope to maintain a minimum 6" asphalt depth at the inside edge of the slab.
- 3. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finish grade. Vary the Junction Slab slope to maintain a minimum 1'-6" thickness at the inside edge of the slab.
- 4. Minimum length of Junction Slab between expansion joints is 30'-0" for 36" Single-Slope or 60'-0" for 42" Single-Slope.
- 5. Contractor to maintain stability of precast coping prior to junction slab completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 1.5" minimum concrete cover.
- 6. 2" cover allows for $\frac{1}{2}$ " variance due to slip forming.

Concret GFRP (exclud Additio Joint (D

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CONCRETE BARRIER/JUNCTI - WALL COPING (FRF



ESTIMATED QUANTITIES FOR C-I-P						
ITEM	UNIT	QUANTITY (36")	QUANTITY (42")			
te	CY/LF	0.376	0.420			
es Bars 6C & 6F)	LF/LF	69.42	72.41			
nal Reinf. @ Expansion Dowels)	LF	8.00	8.00			

(The above concrete quantities are based on a max. superelevation of 6.25%)

SINGLE-SLOPE	CONCRET	E BARRIERS
ION SLAB	INDEX	SHEET
$\mathfrak{D}_{\mathfrak{f}}$	521-611	3 of 4

		FRP BEND	ING DIAGF	RAMS				
		FRP RE	INFORCIN	G				
			LEN	IGTH				
MARK	SIZE	PRECAST FOR SINC	COPING	C-I-P C FOR SINC	COPING GLE-SLOPE	$\frac{(36'')}{(42'')} = \frac{4''}{5''}$	<u>36")</u> 42")	
		(36")	(42")	(36")	(42")	<u>₹</u>		
А	6	5'-3''	5'-5"	7'-10''	8'-0''			+ +
В1	5	11'-6"	9'-6"	AS REQD.	AS REQD.			2
B2	5	AS REQD.	AS REQD.	AS REQD.	AS REQD.			
С	6	4'-10''	4'-10''	N/A	N/A			
F	6	4'-10''	4'-10''	4'-10''	4'-10''		ν	
L	5	4'-5"	4'-5''	4'-5''	4'-5''		4	
Р	4	2'-7"	2'-7''	2'-7''	2'-7"			
5	5	11'-6"	N/A	AS REQD.	N/A	<u>+ </u>		
5	6	N/A	9'-6''	N/A	AS REQD.			
Τ1	5	6'-1''	N/A	6'-1''	N/A	7117		
Т 1	6	N/A	9'-6"	N/A	9'-6"			
Т2	5	6'-1''	N/A	6'-1''	N/A	("9E) 9" (36")	6"	
Τ2	6	N/A	6'-1''	N/A	6'-1''	11" (42")	1	F=-
U	7	4'-4''	4'-6"	4'-4''	4'-6"	STIRRUP BAR 5V1	STIRRUP BAR 5V2	STIRRL
V 1	5	4'-6''	5'-2''	4'-6''	5'-2''		GUARDRAIL	TRANSITION BARS
V2	5	4'-3"	4'-8''	4'-3''	4'-8"		(<u>36")</u> (42")	
V3	5	4'-2''	4'-2''	4'-2"	4'-2''		4" (10"	
V4	5	3'-11"	3'-11"	3'-11''	3'-11"			
1" Ø Dowel	Smooth Bar	2'-0''	2'-0"	2'-0"	2'-0''	3'-0"	3'-0" (36")	7 1/2"
							<i>6'−6" (42")</i>	
						BAR II		
								1, I
						3'-0"	3'-0"	I I
								STIRRUP BAR
						BAR T2		
REINFOR	CING STE	EL NOTES: ons in the be	ndina diaaran	ns are out to	out	-	See Table	2'-0''
2. All r 3. Lap	einforcing splices for	at expansion	and open joi	nts will have ninimum of 2'-	a 1.5" minimum 2".	ver		
4. For 5. The	Precast Co Contractor	pings only, la may use eith	p splice Bars er full length	5 6A with Bars 7 Bars 7A or 1	5 6C. Lap splice ap splice with	will be a minimum of 2'-5". BARS 6A, 5B1	, 5B2, 6C, 6F, 5S, & 6S	1" Ø DOWEL
for 6. Cont	C-I-P Copi ractor may	ngs. use a single	#5 stirrup i	n lieu of two	bars for 5P and	571.		
7. FRP	Bars can'i	not be field b	ent.					
							1	
LAST REVISION	VOISI	.KIPTION:				FDOT FY 2024-25	CONCRETE	BARRIER/JUN
11/01/2						STANDARD PLANS	- W	ALL COPING

TE BARRIER/JUNCTION SLAB - WALL COPING (FRP)

5"

9"

STIRRUP BAR 4P

1" Ø DOWEL





 > DESCRIPTION:	





REVISION








DETAIL "B"

(Showing Locations of $\frac{1}{2}$ " V-Grooves and $\frac{3}{4}$ " Preformed Expansion Joint Filler)



END TRANSITION ELEVATION FOR 32" VERTICAL SHAPE (Guardrail Not Shown For Clarity)

ESTIMATED CONCRETE BARRIER QUANTITIES			
		QUAN	ΙΤΙΤΥ
ITEM	UNIT	32"	42"
Concrete	CY/LF	0.095	0.145
Reinforcing Steel	LB/LF	23.38	28.33

VERTICAL SHAPE

SIDEWALK	INDEX	SHEET
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.4/2023 2:19:

10/4/.



Anchor Bolt design is based on the standard Roadway Aluminum Light Pole

Coating: Galvanize all Nuts, Bolts Washers, and plates in accordance with ASTM F2329. 3. The Contractor is responsible for ensuring the anchor bolt design is compatible with the light pole base plate. Modifications to the anchor bolt design shown must be signed and sealed by the Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.

6. The cost of anchor bolts, nuts, washers and anchor plates will be included in the Bid Price for Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Concrete Barrier or Concrete Parapet that the pedestal is

8. Slip Forming Method of construction requires the Engineer's approval within the limits shown. 9. Reinforcing shown for light pole pedestals is in addition to typical reinforcing for Junction

11. Pedestal may be precast in one section with Coping. Minimum Precast Coping section length is 10 ft. or 12 ft for combination Precast Concrete Barrier and Coping section.

13. Unless otherwise noted, Concrete Barrier (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other Concrete Barriers or pedestrian/bicycle railings are similar.

BO	LTS (1"	Dia.)
of Pedestal Height (Ft.)*		
ninaire Mounting Height		
Ft.	45 Ft.	50 Ft.
5	75	75
5	75	75
5	75	45**
5	75	25**

greater than the height shown and less than 75'.

	INDEX	SHEET
LL COPING	521-650	1 of 3



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DESCRIPTION:



STANDARD PLANS

LIGHT POLE PEDESTAL - WAY

INDEX	SHEET
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ESTIMATED QUANTITIES			
ITEM	UNIT	QUANTITY	
edestal)	СҮ	0.926	
nickened Junction Slab)	СҮ	1.222	
Steel	LB	334.09	

	INDEX	SHEET
LL COPING	521-650	3 of 3

GENERAL NOTES:

- 1. Construct sidewalks in accordance with Specification 522. Use 6" concrete for Sidewalks and Curb Ramps Located within Curb Returns (See Plan View). Install all other concrete with thickness as shown, unless otherwise detailed in the Plans.
- 2. Include detectable warnings on sidewalk curb ramps in accordance with Index 522-002.
- 3. For Driveways see Index 522-003.
- 4. Bond breaker material can be any impermeable coated or sheet membrane or preformed material having a thickness of not less than 6 mils and not more than $\frac{\eta_2}{2}$.
- 5. Construct sidewalks with Edge Beam through the limits of any surface mounted Pedestrian/Bicycle Railing or Pipe Guiderail shown in the plans. (See RAILING DETAIL)

В

В С B





В

B



SAWED JOINTS

LONGITUDINAL SECTION

LEGEND:

- A- 1/2" Expansion Joints (Preformed Joint Filler) between the sidewalk and; *driveways, sidewalk-intersections, and all other fixed objects (e.g. drainage inlets and utility poles).*
- B- ¹/₈" Dummy Joints, Tooled
- $C \frac{1}{8}$ " Formed Open Joints
- D- $\frac{3}{16}$ " Saw Cut Joints, $1\frac{1}{2}$ " Deep (within 96 hours) Max. 5' Centers
- $E-\frac{3}{16}"$ Saw Cut Joints, $1\frac{1}{2}"$ Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'
- $F = \frac{1}{2}$ " Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.
- G- Cold Joint With Bond Breaker, Tooled

SIDEWALK JOINTS ==





STANDARD PLANS



GENERAL NOTES:

1. Cross Slopes and Grades:

- A. Sidewalk, ramp, and landing slopes (i.e. 0.02, 0.05, and 1:12) shown in this Index are maximums. With approval of the Engineer, provide the minimum feasible slope where the requirements cannot be met.
- B. Landings must have cross-slopes less than or equal to 0.02 in any direction.
- C. Maintain a single longitudinal slope along each side of the curb ramp. Ramp slopes are not required to exceed 15 feet in length.
- D. Joints permitted at the location of Slope Breaks. Otherwise locate joints in accordance with Index 522-001. No joints are permitted within the ramp portion of the Curb Ramp.

2. Curb, Curb and Gutter and/or Sidewalk:

- A. Refer to Index 522-001 for concrete thickness and sidewalk details.
- B. Remove any existing curb, curb and gutter, or sidewalk to the nearest joint beyond the curb transition or to the extent that no remaining section is less than 5 feet long.
- C. Width of Curb Ramp is 4'-0" minimum. Match sidewalk or Shared Use Path width as shown in the Plans.
- 3. Curb Ramp Alpha-Identification:
- A. Sidewalk curb ramp alpha-identifications (e.g. CR-A) are provided for reference purposes in the Plans.
- B. Alpha-identifications CR-I and CR-J are intentionally omitted.
- 4. Detectable Warnings:
- A. Install detectable warnings in accordance with Specification 527.
- B. Place detectable warnings across the full width of the ramp or landing, to a minimum depth of 2 feet measured perpendicular to the curb line and no greater than 5 feet from the back of the curb or edge of pavement.
- C. If detectable warnings are shown in the Plans on slopes greater than 5%, align the truncated domes with the centerline of the ramp; otherwise, the truncated domes are not required to be aligned.









12/2024



LAST REVISION













Flared Driveway (6" Thick Concrete)

Sidewalk Thru Driveway (6" Thick Concrete)

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EWAIS	522-003	2 of 4







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JDDING	524-001	1 of 3







NOTES

- 1. Work this Index with the Noise Wall Data Tables, and Wall Control Drawings in the Plans. A. Prestressed concrete posts with equivalent strength resistance may be substituted for conventionally reinforced precast posts shown in this index when approved as part of a Producer's Quality Control Plan.
 - B. Producer shop drawings for prestressed concrete post designs must be approved by the State Structures Design Office prior to inclusion in the Quality Control Plan.
- 2. Construct Noise Walls in accordance with the requirements of Specification Section 534, and Augers Cast Piles in accordance with Specification Section 455.
- 3. Field verify the location of all overhead and underground services shown in the Wall Control Drawings.
- 4. Wall Height is the nominal height of the walls above finished grade. The Wall Embedment Depth for design is 1'-0". The actual embedment depth may vary plus or minus 6" along the length of the wall.
- 5. Post Spacing in this Index are nominal, and are measured from centerline to centerline of the auger cast piles. Actual post spacing may vary as shown in the Wall Control Drawings.

6. Panels:

- A. The sum of the individual stacked panel heights is the Wall Height plus 1'-0" (embedment depth).
- B. Where special graphics are required, locate the horizontal panel joints outside of the graphics. Where possible, hold horizontal panel joints at a constant elevation.
- C. Side Installed Panels are only permitted when reduced overhead clearance between posts prohibits installing panels from the top.
 - 1. For Flush Face panels, install panel into posts from the back face of the wall. Recessed panels may be installed from the back or front face of the wall.
 - 2. After panels are installed and centered between posts, grout between both panel ends and the adjoining posts (see Sheets 4 and 5 for details).
- D. Individual panel heights should be between 6'-0" and 12'-0" tall. The minimum panel height is 4'-0" and may be used where overhead clearance is limited, or where graphic panels are required on shorter walls.

7. Concrete And Grout:

- A. Concrete Class and Compressive Strength for:
 - 1. Precast Panels, Posts, and Post Caps: Class IV
 - 2. Cast-In-Place Collars: Class IV
- B. Minimum Compressive Strength for form removal and handling of posts and panels:
 - 1. 2,500 psi for horizontally cast post and panels
 - 2. 2,000 psi for vertically cast panels or when tilt-up tables are used for horizontally cast panels.
- C. Grout for Auger Cast Piles:
 - 1. Maximum Working Compressive Strength = 2,000 psi
 - 2. Minimum 28 day strength = 5,000 psi
- 8. Reinforcing Steel:
 - A. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
 - 1. Post Stirrups Tie at all four corner bars and at every third interior bar intersection
 - 2. Pile Stirrups Tie to the main vertical reinforcing at alternate intersections for circular configurations and at the four corners and at every third interior bar intersection for rectangular configurations.
 - B. Provide 2" concrete cover unless noted otherwise.
- 9. Casting Tolerances for precast panels and posts:
 - A. Overall Height and Width: $+/-\frac{1}{4}$ "
 - B. Thickness: +/- 1/4"
 - C. Plane of side mold: +/- 1/16"
 - D. Openings: $+/-\frac{1}{2}$ "
 - E. Out of Square: 1/8" per 6 ft., but not more than 3/8"total along any side
 - F. Warping: 1/16" per foot distance to nearest corner
 - G. Bowing: 1/240 panel dimension
 - H. Surface Smoothness for Type "A" Smooth Surface Texture Option: +/- 1/16"

- 10. Provide Plain or Fiber Reinforced Bearing Pads meeting the requirements of Specification Section 932 for Ancillary Structures. A. For Collar Bearing Points provide:
 - 1. $4''x 4''x \frac{1}{2}''$ Fiber Reinforced Pads;
 - 2. Plain Pads may be substituted for Fiber Reinforced Pads when sufficient bearing area is available on the concrete collar for the following:
 - a. $10^{'}$ Post Spacing: $4''x 4''x \frac{1}{2}''$
 - c. 20' Post Spacing and Wall Height \geq 17 feet: 4"x 5"x $\frac{1}{2}$ "
 - B. At panel bearing points between stacked panels, use Plain or Fiber Reinforced Bearing Pads.

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b. 20' Post Spacing and Wall Height < 17 feet: $4''x 4''x \frac{1}{2}''$

GENERAL NOTES

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DESCRIPTION: LAST REVISION 11/01/13



FY 2024-25 STANDARD PLANS

NOISE WALLS - (PRECA

TEXTURE	OPTIONS
---------	---------

AST)	INDEX	SHEET
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DESCRIPTION: LAST REVISION 11/01/14



FY 2024-25 STANDARD PLANS

NOISE WALLS - (PRECA

Single layer flat surface attached to form liner for casting smooth areas of wall design. See plans for project specific graphic drawings. Joints between flat surface and form liner

GRAPHICS & TEXTURE DETAILS

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REVISION 11/01/15



11/01/13



CASE 2 (Exterior Angle)

TYPICAL PANEL DETAILS

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DRAINAGE HOLE DETAILS

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90° CORNER POST DETAILS SHEET 11 of 16








NOISE WALLS - (PRECAST)









PILE/POST ELEVATION

* See Sheet 1, Note 4.

	TABLE 1A - TABLE OF POST REINFORCING STEEL											TAE	BLE 1B	- PIL	E LENG	GTHS	(Feet)	- WIN	ID SPE	ED =	130 M	РН									
	POST LENGTHS WIND SPEED = 130 MPH							10'-0" POST SPACING 20'-0" POST SPACING																							
NOMINAL WALL					10 POST S	10'-0" SPACING		20'-0" POST SPACING			NOMINAL WALL		H-P	OSTS			CORNEF	R POSTS			H-P	OSTS			CORNEF	? POSTS					
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	BA	ARS B	BARS D	Bi	ARS E	BARS A	BA	NRS B	BARS D	BA	ARS E	HEIGHT (Feet)	50.	'L 1	501	IL 2	50	IL 1	50	'L 2	50.	IL 1	501	'L 2	50.	IL 1	5 0i	IL 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		30" ©	36" O	30" ⊘	36" ⊘	30" ⊘	36" ⊘	30" ⊘	36" ©	30" ©	36" ⊘	30" ⊘	36" ©	30" ⊘	36" ⊘	30" ⊘	36" ©
12	13'-0½"	13'-2½"	#4	#4	7'-11"	#4	#4	9'-11"	#5	#5	9'-8''	#6	#6	9'-4"	12	11	10	10	10	11	10	10	10	15	14	13	12	14	13	13	12
13	14'-0 ¹ ⁄2"	$14' - 2^{1/2''}$	#4	#4	10'-11"	#4	#4	10'-11''	#5	#5	9'-8''	#6	#6	9'-4"	13	12	11	10	10	11	10	10	10	15	14	13	13	15	14	13	12
14	15'-0 ¹ ⁄2"	15'-2 ¹ / ₂ "	#4	#4	10'-11"	#5	#5	11'-8"	#6	#6	11'-4"	#7	#7	10'-8''	14	12	11	11	10	12	11	10	10	16	15	14	13	15	14	14	13
15	16'-0½"	16'-2 ¹ /2"	#4	#4	10'-11''	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8''	15	12	12	11	10	12	11	11	10	16	15	15	13	16	15	14	13
16	17'-0 ¹ ⁄2"	17'-2½"	#5	#5	13'-8"	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8''	16	13	12	11	11	12	12	11	10	17	16	15	14	16	15	15	14
17	18'-0 ¹ ⁄2"	18'-2½"	#5	#5	14'-8"	#5	#5	12'-8"	#7	#7	12'-8"	#7	#8	10'-0''	17	13	12	12	11	13	12	11	11	18	16	16	14	17	16	15	14
18	19'-0 ¹ ⁄2"	19'-2½"	#5	#5	14'-8"	#6	#6	14'-4"	#7	#7	12'-8"	#8	#8	12'-0"	18	14	13	12	11	13	12	12	11	18	17	16	15	18	16	15	14
19	20'-0 ¹ ⁄2"	20'-2 ¹ /2"	#5	#5	14'-8"	#6	#6	14'-4"	#7	#8	12'-0"	#8	#9	11'-3"	19	14	13	12	12	14	13	12	11	19	17	16	15	18	17	16	15
20	21'-0 ¹ ⁄2"	21'-2 ¹ ⁄2"	#6	#6	16'-4"	#6	#6	14'-4"	#8	#7	14'-8''	#9	#8	14'-0''	20	14	13	13	12	14	13	12	12	19	18	17	16	19	17	16	15
21	22'-0 ¹ ⁄2"	22'-2 ¹ / ₂ "	#6	#6	16'-4"	#6	#6	14'-4"	#8	#8	14'-0"	#9	#10	12'-4"	21	15	14	13	12	14	13	13	12	20	18	17	16	19	18	17	16
22	23'-0 ¹ ⁄2"	23'-2 ¹ ⁄2"	#6	#6	16'-4"	#7	#7	16'-8"	#8	#9	13'-3"	#10	#9	15'-3''	22	15	14	14	13	15	14	13	12	20	19	18	17	20	18	17	16



TABLE NOTE:

1. Bars D and Bars E are for 45° Corner Posts only.

2. See Contract Plans for project wind speed.

3. Soil 1 = Loose Granular Soil, N = 4 to 9.

Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

PILE DEPTH & REINFORCING SUMMARY

INDEX	SHEET
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TABLE 2A - TABLE OF POST REINFORCING STEEL													ТАБ	BLE 2B	- PILE	E LENG	GTHS	(Feet)	- WIN	ID SPE	EED =	150 M	1РН								
	POST L	ENGTHS					WIND	SPEEL	D = 15	0 MPH	1					10'-0" POST SPACING 20'-0" POST SPACING															
NOMINAL WALL					10' POST S	–0" PACING					20 POST S	'–0" SPACING			NOMINAL WALL		H-P	0STS			CORNER	POSTS			H-P	OSTS			CORNE	R POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	BA	ARS B	BARS D	BA	ARS E	BARS A	BA	ARS B	BARS D	BA	ARS E	HEIGHT (Feet)	50	'L 1	501	L 2	501	L 1	501	IL 2	50	IL 1	50.	IL 2	50	IL 1	501	L 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		30" ⊘	36" ⊘	30" ⊘	36″ ⊘	30" ⊘	36" ⊘	30" ⊘	36" ⊘	30″ ⊘	36" ⊘	30" ⊘	36″ ⊘	30" Ø	36″ ⊘	30" ⊘	36" Ø
12	13'-0 ¹ /2"	13'-2 ¹ /2"	#4	#4	9'-11"	#5	#5	9'-8"	#6	#6	9'-4"	#6	#6	8'-4"	12	12	12	11	10	12	11	11	10	17	15	15	14	16	15	14	13
13	14'-0½"	14'-2 ¹ /2"	#4	#4	9'-11''	#5	#5	10'-8''	#6	#6	9'-4''	#7	#7	8'-8''	13	13	12	11	11	13	12	11	10	17	16	15	14	17	15	15	14
14	15'-0 ¹ / ₂ "	15'-2 ¹ / ₂ "	#5	#5	11'-8"	#5	#5	10'-8"	#7	#7	10'-8"	#7	#7	8'-8''	14	13	12	12	11	13	12	12	11	18	17	16	15	17	16	15	14
15	16'-0 ¹ /2"	16'-2 ¹ /2"	#5	#5	11'-8''	#6	#6	12'-4"	#7	#7	10'-8"	#8	#7	10'-8"	15	14	13	12	11	13	13	12	11	19	17	16	15	18	17	16	15
16	17'-0 ¹ /2"	17'-2 ¹ /2"	#5	#5	11'-8''	#6	#6	12'-4"	#7	#7	10'-8''	#8	#8	10'-0''	16	14	13	13	12	14	13	12	12	19	18	17	16	19	17	16	15
17	18'-0 ¹ /2"	18'-2 ¹ /2"	#6	#6	14'-4"	#6	#6	12'-4"	#7	#8	10'-0"	#9	#8	11'-0"	17	15	14	13	12	14	13	13	12	20	18	17	16	19	18	17	16
18	19'-0 ¹ /2"	$19'-2\frac{1}{2''}$	#6	#6	14'-4"	#7	#7	13'-8"	#8	#8	12'-0"	#9	#10	9'-4"	18	15	14	14	13	15	14	13	12	20	19	18	17	20	18	17	16
20	20'-0'/2''	20'-2'/2''	#6	#6	14'-4"	#7	#/	13'-8"	#8	#9	17-3	#10	#9	12'-3''	19	16	15	14	13	15	14	14	13	21	19	19	1/	20	19	18	17
20	21'-0'2'' 22'-0'/2''	21' - 272'' 27' - 21/2''	#0	#0	14-4	#7	#8	13-0	#9 #9	#9	13-3 12'-A''	#10	#10	11-4	20	10	15	14	13	16	15	14	13	22	20	20	18	21	20	18	17
21	22 - 072 $23' - 0^{1/2''}$	$22 - 2^{1}/2^{2}$	#7	#7	16'-8"	#8	#8	16'-0"	#10	#10	14'-3"	#11	#10	12'-5"	21	17	16	15	14	17	15	14	14	22	21	20	19	21	20	19	18
		T	ABLE 3	3A - T	ABLE (DF PC)ST RE	INFOR	CING	STEEL								TAB	LE 3B	- PILI	E LEN	GTHS	(Feet)	- WIN	ID SPI	EED =	170 M	1PH			
	POST L	T	ABLE 3	3A - T	ABLE (DF PC	DST RE	INFOR	CING 0 = 17	STEEL 0 MPF								T AE 10	BLE 3B	- PILL T SPACI	E LEN(GTHS	(Feet)	- WIN	ID SPI	EED = 20	170 N 1'-0" POS	1PH 57 SPAC	ING		
NOMINAL	POST L	T ENGTHS	ABLE 3	3A - T	ABLE (-0" PACING	DST RE WIND	INFOR SPEEL	CING D = 17	STEEL O MPH	- - 1 20 POST \$	'-0'' 5PACING			NOMINAL WALL		H-P(TAE 10 05T5	BLE 3B	- PILL T SPACI	E LEN (NG CORNEF	GTHS POSTS	(Feet)	- WIN	ID SPE H-PC	EED = 20 0STS	170 N "-0" POS	1PH 57 SPAC.	ING CORNEF	R POSTS	
NOMINAL WALL HEIGHT (Feet)	POST L WITHOUT	T ENGTHS WITH	ABLE 3	ЗА - Т В,	ABLE (10' POST S	-O" PACING BARS	DST RE WIND	INFOR SPEEL	CING $D = 17$ $BARS$	STEEL	- 1 20 POST S	'-0" 5PACING BARS	B	ARS	NOMINAL WALL HEIGHT (Feet)	50	H-P(T AE 10 0ST S 501	BLE 3B '-0'' POS	- PILI T SPACI S0I	E LENG NG CORNEF L 1	GTHS POSTS	(Feet)	- WIN	ID SPI H-P(EED = 20 05T5 50.	170 M 170 POS	1PH ST SPAC. S0	ING CORNEI IL 1	R POSTS	L 2
NOMINAL WALL HEIGHT (Feet)	POST L WITHOUT CAP	T ENGTHS WITH CAP	ABLE 3 BARS A SIZE	BA - T B, SIZE	ABLE (10' POST S ARS B DIM	DF PC -0" PACING BARS D SIZE	DST RE WIND BA SIZE	SPEEL SPEEL	CING D = 17 BARS A SIZE	STEEL O MPF BA SIZE	- 20 POST S ARS B DIM	'-0" 5PACING BARS D SIZE	B, SIZE	ARS E DIM	NOMINAL WALL HEIGHT (Feet)	50 30"	H-P(1L 1 36"	TAE 10 05T5 501 30"	BLE 3B '-0" POS 'L 2 36"	- PILI T SPACI 501 30"	E LENG NG CORNEF L 1 36"	GTHS 2 POSTS 501 30"	(Feet)	- WIN 50. 30"	ID SPI H-P0 IL 1 36"	EED = 20 0STS 50. 30''	170 N "-0" POS IL 2 36"	1PH 5T SPAC. 50 30"	ING CORNEF IL 1 36"	R POSTS 501 30"	L 2 36"
NOMINAL WALL HEIGHT (Feet)	POST L WITHOUT CAP	T ENGTHS WITH CAP	ABLE 3 BARS A SIZE	BA - T Bi SIZE	ABLE (10'' POST S ARS B DIM 'A'	-O" PACING BARS D SIZE	DST RE WIND BA SIZE	ARS E DIM 'A'	CING D = 17 BARS A SIZE	STEEL O MPH BA SIZE	- H POST S ARS B DIM 'A'	'-0" SPACING BARS D SIZE	B/ SIZE	ARS E DIM 'A'	NOMINAL WALL HEIGHT (Feet)	50 30'' 0	H-P0 ″L 1 ∅	TAE 10 0STS \$01 30" ○ 12	BLE 3B '-0" POS 'L 2 36" ⊘	- PILE T SPACI S01 30" ⊘	E LENG NG CORNEF L 1 36" O	GTHS 2 POSTS 501 30″ ⊘	(Feet) /L 2 	- WIN 50. 30'' 0	ID SPE H-P0 IL 1 36″ ⊘	EED = 20 05T5 50. 30″ ⊘	170 № '-0" P05 IL 2 36" ©	1PH 5T SPAC. 50 30″ ⊘	ING CORNEI IL 1 36" ©	R POSTS SOI 30" ⊘	L 2 36" 0
NOMINAL WALL HEIGHT (Feet) 12	POST L WITHOUT CAP	T ENGTHS WITH CAP	ABLE 3 BARS A SIZE #5	3A - T B, SIZE #5	ABLE 10' POST S ARS B DIM 'A' 9'-8'' 10'-8''	DF PC -0" PACING BARS D SIZE #5 #6	DST RE WIND BA SIZE #5 #6	INFOR SPEEL ARS E DIM 'A' 8'-8'' 10'-4''	CING D = 17 BARS A SIZE #6 #7	STEEL O MPH BA SIZE #6 #7	- 20 POST 5 ARS B DIM 'A' 8'-4'' 8'-8''	'-0" 5PACING BARS D SIZE #7 #8	B/ S1ZE #7 #7	ARS E DIM 'A' 7'-8'' 8'-8''	NOMINAL WALL HEIGHT (Feet) 12	50 30" © 14	H-P(1L 1 0 13 13	TAE 10 0STS 30" 0 12 13	BLE 3B '-0" POS 'L 2 36" ⊘ 11 12	- PILI T SPACI 501 30″ ⊘ 13	E LENG NG CORNEF L 1 36" © 12 13	GTHS 2 POSTS 501 30" 0 12 12	(Feet) IL 2 36″ ○ 11	- WIN 50. 30" © 18	<i>ID SPI</i> <i>H−P</i> 0 <i>IL 1</i> <i>36"</i> <i>⊙</i> <i>17</i> <i>18</i>	EED = 20 0STS 50. 30″ ⊘ 16 17	170 № '-0" P05 IL 2 36" ⊘ 15 16	1PH 5T SPAC. 50 30″ ⊘ 18 19	ING CORNEF IL 1 36″ ⊘ 16 17	R POSTS 501 30″ ⊘ 16 16	L 2 36" 0 15 15
NOMINAL WALL HEIGHT (Feet) 12 13 14	POST L WITHOUT CAP 13'-0 ^{1/} 2" 14'-0 ^{1/} 2" 15'-0 ^{1/} 2"	T ENGTHS WITH CAP 13'-2 ¹ /2" 14'-2 ¹ /2" 14'-2 ¹ /2"	ABLE 3 BARS A SIZE #5 #5 #5	BA - T B/ SIZE #5 #5 #5	ABLE 10'' POST 5 ARS B DIM 'A' 'A' 9'-8'' 10'-8'' 10'-8''	-0" PACING BARS D SIZE #5 #6 #6	DST RE WIND BA SIZE #5 #6 #6	SPEEL ARS DIM 'A' 8'-8" 10'-4" 10'-4"	CING D = 17 BARS A SIZE #6 #7 #7	STEEL 0 MPF BA SIZE #6 #7 #7	- - POST 5 ARS B DIM 'A' 8'-4'' 8'-8'' 8'-8''	'-0'' 5PACING BARS D SIZE #7 #8 #8	B/ SIZE #7 #7 #8	ARS E DIM 'A' 7'-8" 8'-8" 8'-0"	NOMINAL WALL HEIGHT (Feet) 12 13 14	50 30" © 14 14 15	H-P0 7L 1 Ø 13 13 14	TAE 10 05TS \$30" 0 12 13 13	BLE 3B '-0" POS 'L 2 36" ⊘ 11 12 12	- PILE T SPACI SOI 30" ⊘ 13 14 14	E LEN NG CORNEF L 1 36" © 12 13 13	GTHS POSTS SOI 30'' 0 12 12 13	(Feet) [L 2 36" ⊘ 11 11 12	- WIN 50 30" © 18 19 20	<i>ID SPt</i> <i>H−P</i> (<i>IL 1</i> <i>36''</i> ⊘ <i>17</i> <i>18</i> <i>18</i>	EED = 20 0STS 50. 30″ ⊘ 16 17 18	170 № '-0" P05 IL 2 36" ⊘ 15 16 16	1PH 57 SPAC. 50 30″ ⊘ 18 19 19	ING CORNEI IL 1 36″ ⊘ 16 17 18	R POSTS S01 30″ ⊘ 16 16 16 17	L 2 36" 0 15 15 16
NOMINAL WALL HEIGHT (Feet) 12 13 14 15	POST L WITHOUT CAP 13'-0 ¹ /2" 14'-0 ¹ /2" 15'-0 ¹ /2" 15'-0 ¹ /2"	T ENGTHS WITH CAP $13'-2^{\frac{1}{2}''}$ $14'-2^{\frac{1}{2}''}$ $15'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$	ABLE 3 BARS A SIZE #5 #5 #5 #5 #5 #6	3A - T Br SIZE #5 #5 #5 #5 #6	ABLE O 10' POST POST S ARS B DIM 'A' 9'-8'' 10'-8'' 10'-8'' 10'-8'' 10'-8'' 12'-4''	-0" PACING BARS D SIZE #5 #6 #6 #6	DST RE WIND 84 84 84 84 84 84 84 84 84 84 84 84 84	TINFOR SPEEL ARS E DIM 'A' 8'-8'' 10'-4'' 10'-4'' 10'-4''	CING D = 17 BARS A SIZE #6 #7 #7 #8	STEEL 0 MPH BA SIZE #6 #7 #7 #7	- - - - - - - - - - - - - -	'-0" 5PACING BARS D SIZE #7 #8 #8 #8 #9	B) S1ZE #7 #7 #8 #8	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15	50 30" ⊘ 14 15 15	H-P0 "L 1 © 13 13 14 14	TAE 10 05T5 \$\$01 30" 0 12 13 13 14	BLE 3B '-0" P05 'L 2 36" ⊙ 11 12 12 12 13	- PILE T SPACI SOI 30" ⊘ 13 14 14 14 15	E LEN NG CORNEF L 1 36" © 12 13 13 13	GTHS 2 POSTS 501 30″ ○ 12 12 13 13	(Feet) 1L 2 36" ⊘ 11 11 12 12	- WIN 50. 30" © 18 19 20 21	<i>ID SPE</i> <i>H−P</i> (<i>IL 1</i> <i>36''</i> ⊘ <i>17</i> <i>18</i> <i>18</i> <i>18</i> <i>19</i>	EED = 20 0STS \$0. 30″ ∅ 16 17 18 18	170 M "-0" P05 IL 2 36" ⊘ 15 16 16 16 17	1PH 57 SPAC 50 30" ⊘ 18 19 19 19 20	ING CORNEF IL 1 36" ○ 16 17 18 18	R POSTS 501 30" ⊘ 16 16 16 17 18	L 2 36" 0 15 15 16 16 16
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16	POST L WITHOUT CAP 13'-0 ^{1/2} " 14'-0 ^{1/2} " 15'-0 ^{1/2} " 16'-0 ^{1/2} " 16'-0 ^{1/2} "	T ENGTHS WITH CAP $13'-2^{\frac{1}{2}''}$ $14'-2^{\frac{1}{2}''}$ $15'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$	ABLE 3 BARS A SIZE #5 #5 #5 #5 #6 #6	BA - T B, SIZE #5 #5 #6 #6	ABLE 10'' POST 5 ARS B DIM 'A' '9'-8'' 10'-8'' 10'-8'' 10'-8'' 12'-4'' 12'-4''	-0" PACING BARS D SIZE #6 #6 #6 #6	DST RE WIND SIZE #5 #6 #6 #6 #6 #7	INFOR SPEEL ARS DIM 'A' 8'-8" 10'-4" 10'-4" 10'-4" 10'-4" 11'-8"	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8	STEEL O MPF BA SIZE #6 #7 #7 #7 #7 #8	- - - - - - - - - - - - - -	'-0'' 5PACING BARS D \$IZE #7 #8 #8 #9 #9 #9	B, SIZE #7 #7 #8 #8 #8 #10	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0" 8'-4"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16	50. 30" © 14 14 15 15 15 16	H-P0 1L 1 0 13 13 14 14 14 15	TAE 10 0STS 30" 0 12 13 14 14	BLE 3B '-0" POS 'L 2 36" ⊘ 11 12 12 13 13	- PILI T SPACI SOI 30" © 13 14 14 14 15 15	E LENG NG CORNEF L 1 36" © 12 13 13 13 14 14	GTHS 2 POSTS 30" 0 12 12 13 13 13 14	(Feet) 1L 2 36" ⊘ 11 11 12 12 13	- WIN 50. 30" © 18 19 20 21 21 21	<i>ID SPI</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36</i> ″ ⊘ <i>17</i> <i>18</i> <i>18</i> <i>19</i> <i>20</i>	EED = 20 0STS 50. 30″ ⊘ 16 17 18 18 18 19	170 M '-0" P05 1L 2 36" ⊘ 15 16 16 17 17	1PH 57 SPAC. 50 30″ ⊘ 18 19 19 20 21	ING CORNEI IL 1 36" ⊘ 16 17 18 18 18 19	R POSTS SOI 30″ ⊘ 16 16 16 17 18 18 18	L 2 36" © 15 15 16 16 17
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	T ENGTHS WITH CAP $13'-2^{\frac{1}{2}''}$ $14'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $18'-2^{\frac{1}{2}''}$	ABLE 3 BARS A SIZE #5 #5 #5 #6 #6 #6	BA - T BA SIZE #5 #5 #6 #6 #6 #6	ABLE 0 POST 5 ARS 8 DIM 'A' 'A' 9'-8" 10'-8" 10'-8" 12'-4" 12'-4" 12'-4" 12'-4"	-0" PACING BARS D SIZE #5 #6 #6 #6 #7 #7	DST RE WIND BA SIZE #6 #6 #6 #7 #7	ARS E DIM 'A' 8'-8'' 10'-4'' 10'-4'' 10'-4'' 10'-4'' 11'-8'' 11'-8''	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8 #8 #9	STEEL 0 MPH 84 51ZE #6 #7 #7 #7 #8 #8	- - - - - - - - - - - - - -	'-0" 5PACING BARS D SIZE #7 #8 #8 #9 #9 #9 #10	B/ SIZE #7 #7 #8 #8 #10 #9	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0" 8'-4" 10'-3"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17	50 30″ ⊘ 14 15 15 16 16 16	<i>H−P</i> (<i>JL</i> 1 <i>36''</i> <i>○</i> <i>13</i> <i>13</i> <i>14</i> <i>14</i> <i>14</i> <i>15</i> <i>15</i>	TAE 10 05TS \$01 30" 0 12 13 14 14 15	BLE 3B '-0" POS 'L 2 36" ⊘ 11 12 12 13 13 13 14	- PILE T SPACI SOI 30" ⊘ 13 14 14 15 15 15 16	E LEN0 NG CORNEF L 1 ○ 12 13 13 14 14 14 15	GTHS 2 POSTS SOI 30" ○ 12 12 13 13 14 14 14	(Feet) <i>IL 2</i> <i>36</i> ″ ⊘ <i>11</i> <i>12</i> <i>12</i> <i>13</i> <i>13</i>	- WIN 50. 30" © 18 19 20 21 21 21 22	<i>ID SPE</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36"</i> ⊘ <i>17</i> <i>18</i> <i>18</i> <i>19</i> <i>20</i> <i>20</i> <i>20</i>	EED = 20 05T5 50. 30'' ⊘ 16 17 18 18 18 19 19	170 M '-0" P05 1L 2 36" ⊘ 15 16 16 17 17 18	ЛРН 5T SPAC. 50 30" 0 18 19 19 20 21 21	ING CORNEI IL 1 36" ⊘ 16 17 18 18 19 20	R POSTS SOI 30" ⊘ 16 16 16 17 18 18 18 18 19	L 2 36" 0 15 15 16 16 17 17
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18	$\begin{array}{c} POST \ L\\ \hline\\ WITHOUT\\ CAP\\ \hline\\ 13'-0^{1/2''}\\ 14'-0^{1/2''}\\ 14'-0^{1/2''}\\ 15'-0^{1/2''}\\ 15'-0^{1/2''}\\ 16'-0^{1/2''}\\ 18'-0^{1/2''}\\ 18'-0^{1/2''}\\ 19'-0^{1/2''}\\ \end{array}$	T ENGTHS WITH CAP $13'-2^{1/2''}$ $14'-2^{1/2''}$ $15'-2^{1/2''}$ $15'-2^{1/2''}$ $16'-2^{1/2''}$ $18'-2^{1/2''}$ $18'-2^{1/2''}$ $19'-2^{1/2''}$	ABLE 3 BARS 4 SIZE #5 #5 #5 #6 #6 #7 #7	BA - T B, SIZE #5 #5 #6 #6 #6 #6 #6 #7	ABLE 0 POST 5 ARS 0 DIM 'A' 9'-8" 10'-8" 10'-8" 12'-4" 12'-4" 12'-4" 13'-8" 13'-8"	DF PC -0" PACING BARS D SIZE #6 #6 #6 #7 #7 #7	DST RE WIND 84 84 84 84 84 84 84 84 84 84 84 84 84	ARS E DIM 'A' 8'-8" 10'-4" 10'-4" 10'-4" 10'-4" 11'-8" 11'-8" 11'-0"	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8 #9 #9 #9	STEEL O MPH BA SIZE #6 #7 #7 #7 #7 #8 #8 #8 #8 #10	- - - - - - - - - - - - - -	'-0" FACING BARS D SIZE #7 #8 #8 #8 #9 #9 #10 #10	B) SIZE #7 #7 #8 #8 #10 #9 #11	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0" 8'-4" 10'-3" 8'-5"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18	50 30″ ⊘ 14 14 15 15 16 16 16 17	<i>H</i> - <i>P</i> (<i>J J</i> <i>J J</i> <i>J</i> <i>J J</i> <i>J J</i> <i>J</i> <i>J J</i> <i>J J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i> <i>J</i>	TAE 10 0STS 30" 0 12 13 14 15 15	BLE 3B '-0" P05 'L 2 36" ⊙ 11 12 12 13 13 14 14 14	- PILE T SPACI SOI 30" ⊘ 13 14 14 15 15 16 16 16	E LENG NG CORNEF L 1 36" © 12 13 13 13 14 14 14 15 15	GTHS 2 POSTS 30" 0 12 12 13 13 13 14 14 14 15	(Feet) IL 2 36" ⊘ 11 11 12 12 13 13 14	- WIN 50. 30" 0 18 19 20 21 21 21 22 23	<i>ID SPE</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36</i> " ⊘ <i>17</i> <i>18</i> <i>18</i> <i>18</i> <i>19</i> <i>20</i> <i>20</i> <i>20</i> <i>21</i>	EED = 20 0STS 0 30″ 0 16 17 18 18 18 19 19 20	170 M r'-0" POS IL 2 36" ⊘ 15 16 16 16 17 17 18 19	1PH 5T SPAC. SO 30" 0 18 19 19 20 21 21 22	ING CORNEH IL 1 36" ⊘ 16 17 18 18 19 20 20 20	R POSTS S01 30" ○ 16 16 17 18 18 19 19 19	L 2 36" © 15 15 16 16 17 17 18
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19	$\begin{array}{c} POST \ L\\ WITHOUT\\ CAP\\ \hline 13'-0^{\frac{1}{2}''}\\ 14'-0^{\frac{1}{2}''}\\ 15'-0^{\frac{1}{2}''}\\ 16'-0^{\frac{1}{2}''}\\ 16'-0^{\frac{1}{2}''}\\ 18'-0^{\frac{1}{2}''}\\ 19'-0^{\frac{1}{2}''}\\ 20'-0^{\frac{1}{2}''}\\ \hline 20'-0^{\frac{1}{2}''}\\ \end{array}$	T ENGTHS WITH CAP $13'-2^{\frac{1}{2}''}$ $14'-2^{\frac{1}{2}''}$ $15'-2^{\frac{1}{2}''}$ $15'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $18'-2^{\frac{1}{2}''}$ $19'-2^{\frac{1}{2}''}$ $20'-2^{\frac{1}{2}''}$	ABLE 3 BARS A SIZE #5 #5 #5 #6 #6 #6 #6 #7 #7	BA - T B/ SIZE #5 #5 #6 #6 #6 #7 #7	ABLE 0 POST 5 ARS 0 B 0 DIM 'A' '9'-8'' 10'-8'' 10'-8'' 10'-8'' 12'-4'' 12'-4'' 13'-8'' 13'-8''	DF PC -0" PACING BARS D SIZE #6 #6 #6 #6 #7 #7 #7 #7 #7	DST RE WIND SIZE #5 #6 #6 #7 #8 #7	ARS E DIM 'A' 8'-8" 10'-4" 10'-4" 10'-4" 10'-4" 11'-8" 11'-8" 11'-8" 11'-8"	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8 #9 #9 #9 #10	STEEL O MPF BA SIZE #6 #7 #7 #7 #7 #7 #7 #8 #8 #10 #10	- - - - - - - - - - - - - -	'-0'' 5PACING BARS D \$IZE #7 #8 #8 #9 #9 #10 #10 #11	B/ SIZE #7 #7 #8 #8 #10 #9 #11 #11	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0" 8'-4" 10'-3" 8'-5" 10'-5"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19	50. 30″ ⊘ 14 15 15 16 16 16 17 17	H-P0 12 1 36'' 0 13 13 14 14 15 15 16 16 16	TAE 10 0STS 30" 0 12 13 14 15 15 15	BLE 3B '-0" POS 'L 2 36" ⊘ 11 12 12 13 13 13 14 14 14 14	- PILE T SPACI SOI 30" ⊘ 13 14 14 15 15 15 16 16 16 17	E LENG NG CORNEF L 1 36" © 12 13 13 13 14 14 15 15 16	GTHS POSTS SO 30" 0 12 12 13 13 14 14 14 15 15	(Feet) ¹ L 2 ^{36"} ⊘ ¹¹¹ ¹¹ ¹² ¹³ ¹³ ¹⁴ ¹⁴	- ₩IN 50 30" © 18 19 20 21 21 21 21 22 23 23 23	<i>ID SPI</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36</i> ″ ⊘ <i>17</i> <i>18</i> <i>19</i> <i>20</i> <i>20</i> <i>21</i> <i>22</i>	EED = 20 0STS 50. 30″ ⊘ 16 17 18 18 18 19 19 20 20 21	170 N r - 0" P05 IL 2 36" ○ 15 16 16 17 17 18 19 19	1PH 5T SPAC. 50 30" ⊘ 18 19 19 20 21 21 21 22 23	ING CORNEI IL 1 36" ⊘ 16 17 18 18 19 20 20 21	R POSTS SOI 30'' ⊘ 16 16 16 17 18 18 18 19 19 19 20	L 2 36" © 15 15 16 16 17 17 18 18 18
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19 20	$\begin{array}{c} POST \ L\\ WITHOUT\\ CAP\\ \hline \\ 13'-0^{\frac{1}{2}''}\\ 14'-0^{\frac{1}{2}''}\\ 14'-0^{\frac{1}{2}''}\\ 15'-0^{\frac{1}{2}''}\\ 15'-0^{\frac{1}{2}''}\\ 16'-0^{\frac{1}{2}''}\\ 17'-0^{\frac{1}{2}''}\\ 18'-0^{\frac{1}{2}''}\\ 19'-0^{\frac{1}{2}''}\\ 20'-0^{\frac{1}{2}''}\\ 21'-0^{\frac{1}{2}''}\\ \end{array}$	T ENGTHS WITH CAP $13'-2^{\frac{1}{2}''}$ $14'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $16'-2^{\frac{1}{2}''}$ $19'-2^{\frac{1}{2}''}$ $20'-2^{\frac{1}{2}''}$ $21'-2^{\frac{1}{2}''}$	ABLE 3 BARS 4 SIZE #5 #5 #5 #6 #6 #7 #7 #7 #7	BA - T B SIZE #5 #5 #6 #6 #6 #6 #7 #7 #7	ABLE 0 POST 5 ARS 8 DIM 'A' 9'-8" 10'-8" 10'-8" 10'-8" 12'-4" 12'-4" 12'-4" 13'-8" 13'-8" 13'-8"	DF PC -0" PACING BARS D SIZE #5 #6 #6 #7 #7 #7 #8 #8	DST RE WIND 84 84 84 84 84 84 84 84 84 84 84 84 84	TINFOR SPEEL ARS E DIM 'A' 8'-8'' 10'-4'' 10'-4'' 10'-4'' 10'-4'' 11'-8'' 11'-8'' 11'-8'' 11'-8'' 11'-0'' 13'-8'' 13'-0''	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8 #8 #9 #9 #10 #10	STEEL O MPH BA SIZE #6 #7 #7 #7 #7 #8 #8 #8 #10 #10 #11	- - - - - - - - - - - - - -	'-0" 5PACING BARS D SIZE #7 #8 #8 #9 #9 #9 #10 #10 #11 #11	B/ SIZE #7 #7 #8 #8 #10 #9 #11 #11 #11 #14	ARS E DIM 'A' 7'-8" 8'-8" 8'-8" 8'-0" 10'-0" 8'-4" 10'-3" 8'-5" 10'-5" 7'-0"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19 20	50 30″ ⊘ 14 15 15 16 16 16 17 17 17 18	H-P(UL 1 $36'' \odot$ 13 13 14 14 15 15 16 16 17	TAE 10 0STS \$01 30" 0 12 13 14 15 15 16	<i>BLE 3B</i> '-0" POS [−] L 2 36" ⊘ 11 12 13 13 13 14 14 14 14 14 15	- PILE T SPACI SOI 30" ⊘ 13 14 14 15 15 16 16 16 16 17 17	E LEN0 NG CORNEF L 1 36" ⊘ 12 13 13 14 14 14 15 15 16 16 16	GTHS 2 POSTS SOI 30" ○ 12 12 13 13 14 14 15 15 15 15	(Feet) <i>IL 2</i> 36" ⊘ 11 11 12 12 13 13 14 14 14 14	- WIN 50. 30" © 18 19 20 21 21 21 21 21 22 23 23 23 24	<i>ID SPE</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36"</i> ⊘ <i>17</i> <i>18</i> <i>19</i> <i>20</i> <i>20</i> <i>20</i> <i>20</i> <i>20</i> <i>20</i> <i>21</i> <i>22</i> <i>22</i>	EED = 20 0575 50. 30" 0 16 17 18 18 19 19 20 21 21	170 N r'-0" P05 IL 2 36" Ø 15 16 16 17 17 18 19 19 20 20	APH 5T SPAC 30" 0 18 19 20 21 21 21 22 23 23	ING CORNEI IL 1 36" ⊘ 16 17 18 18 19 20 20 20 21 21 21	R POSTS SOI Ø 16 16 17 18 18 18 19 19 20 20 20	L 2 36" ⊘ 15 15 16 16 17 17 18 18 19
NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19 20 21	$\begin{array}{c} POST \ L\\ \hline\\ WITHOUT\\ CAP\\ \hline\\ 13'-0^{1/2''}\\ 14'-0^{1/2''}\\ 15'-0^{1/2''}\\ 15'-0^{1/2''}\\ 15'-0^{1/2''}\\ 16'-0^{1/2''}\\ 18'-0^{1/2''}\\ 18'-0^{1/2''}\\ 20'-0^{1/2''}\\ 20'-0^{1/2''}\\ 21'-0^{1/2''}\\ 22'-0^{1/2''}\\ \end{array}$	T ENGTHS WITH CAP $13'-2^{1}/2''$ $14'-2^{1}/2''$ $15'-2^{1}/2''$ $15'-2^{1}/2''$ $16'-2^{1}/2''$ $16'-2^{1}/2''$ $17'-2^{1}/2''$ $18'-2^{1}/2''$ $20'-2^{1}/2''$ $21'-2^{1}/2''$ $22'-2^{1}/2''$	ABLE 3 BARS A SIZE #5 #5 #5 #6 #6 #7 #7 #7 #7	BA - T B/ SIZE #5 #5 #6 #6 #6 #6 #7 #7 #7 #7	ABLE 0 POST 5 ARS 0 DIM 'A' 'A' 9'-8" 10'-8" 10'-8" 10'-8" 12'-4" 12'-4" 12'-4" 13'-8" 13'-8" 13'-8" 13'-8"	OF PC -0" PACING BARS D SIZE #5 #6 #6 #7 #7 #8 #8 #9 *9	ST RE WIND SIZE #6 #6 #6 #7 #8 #7 #8 #7 #8 #7 #8 #7	INFOR SPEED ARS DIM 'A' 8'-8'' 10'-4'' 10'-4'' 10'-4'' 11'-8'' 11'-8'' 13'-0'' 15'-0''	CING D = 17 BARS A SIZE #6 #7 #7 #8 #8 #9 #9 #10 #10 #11	STEEL O MPH BA SIZE #6 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7	- - - - - - - - - - - - - -	O" FACING BARS D SIZE #7 #8 #8 #8 #9 #9 #10 #10 #11 #11 #11 #14	B) SIZE #7 #8 #8 #10 #9 #11 #11 #11 #14 #11	ARS E DIM 'A' 7'-8" 8'-8" 8'-0" 10'-0" 8'-4" 10'-3" 8'-5" 10'-5" 7'-0" 12'-5"	NOMINAL WALL HEIGHT (Feet) 12 13 14 15 16 17 18 19 20 21	50. 30″ ⊘ 14 14 15 15 16 16 17 17 17 18 18 18	H-P0 7L 1 Ø 13 13 14 14 15 15 16 16 16 17 17	TAE 10 0STS SOI 30" 0 12 13 14 15 15 16 16	<i>BLE 3B</i> <i>'-0" POS</i> <i>L 2</i> <i>36"</i> ⊘ <i>11</i> <i>12</i> <i>13</i> <i>14</i> <i>14</i> <i>14</i> <i>14</i> <i>14</i> <i>15</i> <i>15</i> <i>15</i>	- PILE T SPACI SOI 30" ⊘ 13 14 14 15 15 16 16 16 16 17 17 17 18	E LENG NG CORNEF L 1 36" © 12 13 13 13 14 15 15 16 16 16 17	GTHS POSTS SO 30" © 12 12 13 13 14 14 15 15 15 16	(Feet)	- WIN 50. 30" 0 18 19 20 21 21 22 23 23 23 23 24 25	<i>ID SPI</i> <i>H−P</i> 0 <i>IL</i> 1 <i>36"</i> ⊘ <i>17</i> <i>18</i> <i>18</i> <i>19</i> <i>20</i> <i>20</i> <i>20</i> <i>21</i> <i>22</i> <i>22</i> <i>22</i> <i>23</i>	$EED = 20$ 0575 $S0$ $30'' \odot$ 16 17 18 18 19 19 20 21 21 21 22	170 M r'-0" POS IL 2 36" ○ 15 16 16 17 17 18 19 19 20 20 20	ЛРН ST SPAC. 30" 30" 19 19 20 21 21 21 21 23 23 24	ING CORNER IL 1 36" © 16 17 18 18 19 20 20 20 21 21 22	R POSTS S01 30" ○ 16 16 17 18 18 19 19 19 20 20 21	L 2 36" © 15 15 16 16 17 17 18 18 19 19 19

TABLE NOTE:

11/01/16

1. Bars D and Bars E are for 45° Corner Posts only.

2. See Contract Plans for project wind speed.

3. Soil 1 = Loose Granular Soil, N = 4 to 9;

Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

Md

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LAST ODESCRIPTION: REVISION 5





NOISE WALLS - (PRECA

PILE DEPTH & REINFORCING SUMMARY

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GENERAL NOTES:

- 1. Construct Perimeter Walls in accordance with Specification Section 534.
- 2. Choice of either Precast Option or Masonry Option is at the discretion of the Contractor. Contractor must also select the desired foundation type. Modifications to this Index is restricted to those required for geometric needs only.
- 3. Post spacing is measured from centerline to centerline of foundation element. For this Index, posts and foundation elements have been designed for 20 ft. spacings. Use post spacings less than 20 feet only at changes in horizontal alignment, wall terminations or to accommodate steep grades.
- 4. See "Perimeter Wall Data Tables" in the plans for project requirements.
- 5. Field verify the locations of all overhead and underground utilities shown in the Wall Control Drawings.

PRECAST OPTION NOTES:

6. WALL NOTES:

- A. Walls may consist of either a single height panel or two stacked panels. Minimum panel height is 4'-3".
- B. Only when reduced overhead clearance between posts prohibits installation of panels from the top, side-installed panels are allowed. After panel is centered between posts, grout between panel ends and posts.

7. CONCRETE AND GROUT:

- A. Cast-in-Place and Precast Concrete: Class IV
- B. Grout for Auger Cast Piling: Minimum 28 Day Strength = 5000 psi
- C. Minimum Compressive Strength for Form Removal and Handling of Posts, Panels and Precast Spread Footings:
 - i. 2,500 psi for horizontally cast post, panels and precast spread footings.
- ii. 2,000 psi for vertically cast panels or when tilt-up form tables are used for horizontally cast panels.

8. REINFORCING STEEL:

- A. Concrete Cover: $1\frac{1}{2}$ " unless otherwise noted.
- B. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
- i. Post Stirrups Tie at all four corner bars and at every third interior bar intersection.
- *ii.* Pile Stirrups Tie to the main vertical reinforcing at alternate intersections.

9. BEARING PADS:

A. Bearing Pads for Collar or Pedestal Bearing Points and between stacked panels may be either Plain or Fiber Reinforced Neoprene Pads, in accordance with Specification Section 932 for ancillary structures.

10. CASTING TOLERANCES:

- A. Overall Height & Width: $+/-\frac{1}{4}$ "
- B. Thickness: $+/-\frac{1}{4}$ "
- C. Plane of side mold: $+/-\frac{1}{16''}$
- D. Openings: $+/-\frac{1}{2}$ "
- E. Out of Square: 1/3" per 6 ft., but not more than 3/3" total along any side
- F. Warping: $\frac{\eta_{16}}{16}$ per foot distance to nearest corner
- G. Bowing: 1/240 panel dimension

11. PILING:

Construct Auger Cast Piling in accordance with the Plans and Specification Section 455.

MASONRY OPTION NOTES:

12. WALL NOTES:

- A. Inspect construction in accordance with the International Building Code (IBC) Section 17.
- B. Construct masonry walls with 8x8x16 block using a running bond pattern and concave tooled joints.
- C. Make all elevation changes (steps) in footing and top of wall using full height blocks. Make top of wall steps at pilasters exclusively. Footing steps may be made between pilasters as necessary to maintain minimum soil cover.

MASONRY OPTION NOTES (CONT.):

- D. Fully Grout all cells with horizontal or vertical reinforcing bars.
- E. Use reinforcing bar positioners to maintain vertical and horizontal bar placement.
- F. Fully grout first three courses of the wall.
- G. Joint Reinforcement: Use W 1.7 (9mm) galvanized ladder reinforcing spaced at 16" vertically. Provide special accessories for corners, intersections, etc. Joint reinforcing shall be continuous except it shall not pass through vertical masonry control joints. Lap joint reinforcing a minimum of 6".
- H. Construct expansion joints in the foundation at 90 foot maximum intervals, and directly below a wall control joint. I. Dowel Load Transfer Devices will be ASTM A 36 smooth round bars hot-dip galvanized in accordance with Specification
- Section 962. Install Dowel Load Transfer Devices in accordance with Specification Section 350. J. For spread footings, use a walk-behind compactor of at least 600 lbs. in weight. Obtain a minimum density of 95% of the
- maximum dry density as determined by FM 1 T-180. Perform soil density tests at 100 foot intervals. K. Protect walls during construction from soil, grout or mortar stains. Clean wall as work progresses by dry brushing to remove mortar fins and smears before tooling joints.
- L. Use soap and potable water to clean walls. If stain removal is necessary, use a cleaning method indicated in NCMA TEK 8-2A applicable to the type of stain on the exposed surface.
- M. During construction, cover tops of walls, with waterproof sheeting at the end of each day's work, or when construction is not in progress. Extend sheeting a minimum of 2 feet down each side and secure in place.
- N. Comply with Hot Weather Requirements in ACI 530.1.

13. MATERIALS:

- A. Concrete Masonry Units (CMU): Provide normal weight blocks.
- B. Cast-In-Place Concrete: Class II for slightly to moderate aggressive environments or Class IV for extremely aggressive environments
- C. Mortar: Type S meeting requirements of ASTM C1329
- D. Grout: Type S; coarse grout.
- E. Aggregate for Grout: Meet the requirements of ASTM C404 or Specification Section 901 size 8 or 89.

14. STORAGE OF MATERIALS:

- A. Store CMU's on elevated platforms in a dry location or under cover. If units become wet, do not install until they are dry.
- B. Store cementitious materials on elevated platforms, under cover, and in a dry location. Do not use cementitious materials that have become damp or exceeded the manufacturers shelf life.
- C. Store masonry accessories and reinforcing to prevent corrosion and accumulation of dirt and oil.



11/01/17









SECTION B-B





SECTION B-B

PERIMETER WALLS





- 1. Drainage holes may be formed with 4" NPS PVC pipe that
 - may remain in place.
- 2. See Wall Control drawings for number, Type and location/ spacing of drainage holes.





FY 2024-25 STANDARD PLANS

PERIMETER WALLS

DRAINAGE DE	TAILS
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PRECAST OPTION - TYPICAL DETAILS

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SECTION H-H

TABLE 1												
Wind Speed (MPH)	Pile Length	Bars A	Bars P1 thru P6	Bars S1								
130	12'-0''	#5	#3	#4								
150	13'-6''	#5	#3	#4								
170	15'-0"	#6	#3	#4								





10/4/2023





TYPICAL ELEVATION (T-Footing Shown, Trench Footing Similar)

		Tab	ole 2		
Wind Speed Category	Masonry	v Walls		Foundatior	าร
	(8x8)	x16)	Bars	T-Footing	Trench Footing
	Bars V1	SV Spacing	F1 & F2	(W)	Depth (D)
130	#5	2'-8"	#5	4'-4''	5'-6"
150	#5	2'-0"	#5	5'-0"	6'-4"
170	#5	1'-4''	#5	6'-0"	7'-0"

Notes:

11/01/17

- 1. End vertical reinforcing bars $1^{1}\!\!/_{2}^{\prime\prime}$ from top of bond beam blocks and horizontal bars $1\frac{1}{2}$ " from edge of control joints.
- 2. Do not continue horizontal #4 Bond beam reinforcing through control joint.
- 3. Use stainless steel joint stabilizing anchors spaced at 16" vertically at all control joints. Install per manufacturers instructions.
- 4. Seal Control Joints with backer rod and Type "A" silicone sealant (top and both sides).
- 5. See Sheet 10 for Bar placement details.
- 6. For Pilaster Cap Details, see Sheet 2.



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	10/4/2023

DESCRIPTION: LAST REVISION



FY 2024-25 STANDARD PLANS



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7	General Notes;
1	Index Contents
2	General, TL-3 Guardrail - Installed Plan and Elevation
3	Low-Speed, TL-2 Guardrail - Installed Plan and Elevation
4	W-Beam and Thrie-Beam Panel Details
5	Post and Offset Block Details
6	Guardrail Sections – Heights and Adjacent Slopes
7	End Treatment – Approach Terminal Geometry, Parallel
8	End Treatment – Approach Terminal Geometry, Curbed and Double Faced
9	End Treatment – Trailing Anchorage
10	End Treatment - Component Details
11	End Treatment – Controlled Release Terminal (CRT) System
12	Layout for CRT System - Side Roads and Driveways
13	Approach Transition Connection to Rigid Barrier - General, TL-3
14	Approach Transition Connection to Rigid Barrier - General, TL-3 - Curb Connections
15	Approach Transition Connection to Rigid Barrier - Low-Speed, TL-2
16	Approach Transition Connection to Rigid Barrier - Low-Speed, TL-2 - Curb Connections
17	Approach Transition Connection to Rigid Barrier - Details
18	Approach Transition Connection to Rigid Barrier - Double Faced Guardrail
19	Layout to Rigid Barrier – Approach Ends
	Layout to Rigid Barrier – Approach Ends with Double Faced Guardrail
20	Layout to Rigid Barrier – Trailing Ends
	Trailing End Transition Connection to Rigid Barrier
21	Trailing End Transition Connection to Rigid Barrier - Curb Connections
22	Rub Rail Details
23	Pedestrian Safety Treatment - Pipe Rail
	Modified Mount - Special Steel Post for Concrete Structure Mount;
24	Modified Mount – Encased Post for Shallow Mount;
	Modified Mount - Frangible Leave-Out for Concrete Surface Mount
	Barrier Delineators – Post Mounted;
25	Clear Space - Reduced Post Spacing for Hazards;
	5/8" Button-Head Bolt System

GENERAL NOTES:

- 1. INSTALLATION: Construct guardrail in accordance with Specification 536
 - This Index, along with the plans and the manufacturers' drawings on the Approved Products List (APL), is sufficiently detailed for installation of General Guardrail, Low-Speed Guardrail, End Treatment assemblies, and their connecting options shown herein. This precludes requirements for shop drawing submittals unless otherwise specified in the plans.
- 2. COMPATIBILITY: The General Guardrail in this Index is based on the Midwest Guardrail System (MGS) design, with an approximate height of 31" at the top of the Panel (2'-1" mounting height at vertical ζ of Panel) and a midspan panel splice as shown on Sheet 2. Guardrail components included on the APL, which are compatible with this Index, may also be identified as 31" or MGS Guardrail.
- 3. STANDARD COMPONENTS: Standard guardrail components, including posts, panels, and bolt systems, are based on the Task Force 13 Publication: Guide to Roadside Hardware Components (http://tf13.org/Guides/componentGuide/).
- 4. BUTTON-HEAD BOLTS: Install Button-Head Bolts where indicated using bolts, nuts, and washers as defined on Sheet 25. Place washers under nuts against timber posts. Washers are not required at steel post flanges and panel lap splices. Do not place washers between bolt heads and panels, except where otherwise shown in this Index.
- 5. HEX-HEAD BOLTS: Install Hex-Head Bolts where indicated using bolts, nuts, and washers in accordance with material properties of Specification 967. Place washers under nuts.
- 6. MISCELLANEOUS ASPHALT PAVEMENT: Install Miscellaneous Asphalt Pavement where indicated with a tolerance of $\pm \frac{1}{2}$ " depth and in accordance with Specification 339.
- 7. ADJACENT SIDEWALKS & SHARED USE PATHS: When quardrail posts are placed within 4'-0" of a sidewalk or shared use path, use timber posts, or use steel posts only if treated with Pipe Rail as shown on Sheet 23.
- When timber posts are used, one of the following safety treatments is required for the bolt(s) protruding from the back face of the posts.
- a. After tightening the nut, trim the protruding post bolt flush with the nut and galvanize per Specification 562. b. Use post bolts 15" in length and countersink the washer and nut between 1" and $1\frac{1}{2}$ " deep into the back face of the post. c. Use 15" post bolts with sleeve nuts and washers.

When End Treatment posts are within 4'-0" of a sidewalk or shared use path, steel posts are not permitted within the End Treatment segment. Terminate the Pipe Rail outside of End Treatment segments, as noted per Sheet 23.

- 8. NESTED W-BEAM: Where called for in the plans, install two W-Beam Panels mounted flush per location, securing all panels with Button-Head Bolts threaded through aligned slots and holes. 2" Button-Head Bolts are permitted for panel splice locations.
- 9. CONNECTION TO RIGID BARRIER: The connections to Rigid Barrier in this Index only apply to newly constructed bridge Traffic Railing and Concrete Barrier or where the complete Approach Transition Connection to Rigid Barrier shown herein can be installed without conflicting with existing Traffic Railings, structures, or approach slabs.

For connecting guardrail to existing bridge Traffic Railings, see Indexes 536-002, 521-404, and 521-405.

10. CONNECTION TO EXISTING GUARDRAIL: Where a transition to existing guardrail at 27" height is required, linearly transition the new guardrail height over a distance ranging from 25'-0" to 31'-3". Height transitions must occur outside of End Treatment and Approach Transition segments.

Provide an immediate transition to the required midspan panel splice using the available panel options on Sheet 4 (9'-4 $\frac{1}{2}$ " or 15'- $7\frac{1}{2}$ " panel). Alternatively, this transition to midspan panel splice may be achieved by installing a single reduced post spacing of 3'- $1\frac{1}{2}$ " within the new quardrail, immediately adjacent to the connection location.

11. PLANS CALLOUTS: Begin/End Station labels are shown throughout this Index as they correspond to the station and offset callouts specified in the plans.

In the plans, Begin/End Guardrail Station refers to the General TL-3 Guardrail Pay Item, and it may be abbreviated as Begin/End GR. Station. Where the Low-Speed TL-2 Guardrail Pay Item is specifically required, the callout in the plans will then specify Begin/End TL-2 GR. Station.

12. QUANTITY MEASUREMENT: Measure quardrail and corresponding components as defined in Specification 536. The Guardrail length is measured along the centerline of installed Panels, between the points labeled Begin/End Guardrail Station shown on the following Index Sheets and defined in the plans (typically measured from the Ç of the panel's post bolt slots at the approach/trailing ends).



GUARDRAIL

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INSTALLED SECTION

NOTES:

1. GENERAL: Install the General Guardrail configuration where indicated in the plans. This may include tapered segments if called for in the plans.

Use 12'-6" or longer W-Beam Panels. A single 6'-3" Panel may be used at the end of the run to meet the nominal Begin/End Guardrail Sta. requirements.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 24 for additional post mounting options.
- 8. DEFINED SEGMENTS: The General Guardrail shown provides the base configuration, including Post Spacing and splice locations, for Defined Segment modifications where indicated in the plans and using the Guardrail Types, Sections, and/or hardware as shown in this Index (e.g. Double Faced W-Beam, Deep Posts at Slope Breaks, Pipe Rail, Rub Rail, or Reduced Post Spacing for Hazards).

GENERAL, TL-3 GUARDRAIL DETAILS

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INSTALLED SECTION

NOTES:

1. GENERAL: Install the Low-Speed Guardrail configuration where indicated in the plans. Low-Speed Guardrail may include tapered segments if called for in the plans.

Use 12'-6" or 25'-0" W-Beam Panels for normal spans, and use 9'-4¹/₂" Panels for end connections to adjoining segments as shown. A single 6'-3" Panel may be used at the end of the Low-Speed Guardrail run along with a single reduced 6'-3" post spacing to meet the nominal Begin/End Guardrail Sta. required.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the Plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 24 for additional post mounting options.
- 8. RESTRICTIONS: Low-Speed Guardrail segments are not permitted for use with items including, but not limited to, Double Faced W-Beam, Deep Posts at Slope Breaks, Raised Curb, Pipe Rail, and/or Rub Rail.

LOW-SPEED, TL-2 GUARDRAIL DETAILS

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Panel Type	Number of Spaces 'N'	Gauge
6'-3" W-Beam	2	12
9'-4½" W-Beam	3	12
12'-6" W-Beam	4	12
15'-7½" W-Beam	5	12
25'-0" W-Beam	8	12
3'-1½" Thrie-Beam	1	10
6'-3" Thrie-Beam	2	12
12–6" Thrie–Beam	4	12
25-0" Thrie-Beam	8	12
Thrie-Beam Trans.	2	10

PANEL SUMMARY TABLE:

NOTES:

- 1. MATERIALS: Use corrugated steel panels in accordance with Specification 967 and made from either Class A, 12 gauge steel or Class B, 10 gauge steel as specified in the 'Panel Summary Table' above.
- 2. CABLE ANCHOR PLATE BOLT HOLES: Include $\frac{3}{4}$ " Ø Cable Anchor Plate Bolt Holes only where required for installation of the Cable Anchor Plate shown on Sheet 9, 10, & 11.

 $^{29}\!\!\!\!/_{32}$ x $1'\!\!\!/_8$ slots may substitute for the $^3\!\!/_4$ Ø holes shown.

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POST AND OFFSET	BLOCK I	DETAILS
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1. INSTALLATION: Locate Approach Terminals where called for in the plans, with the Post (1) & placed at the Begin/End Guardrail Station indicated in

The Plan Views shown herein are schematic only, showing basic geometry for Approach Terminals listed on the APL. The predefined Length of End Treatment, 'LE', includes the proprietary portion of various Approach Terminals and provides for more consistent planning of assembly installations across the differing Approach Terminal types. Forwardanchoring style Approach Terminals may vary from the planned lengths

Construct Approach Terminals as shown in the APL and in accordance with the manufacturer's unique drawing details, procedures, and specifications.

Install posts in accordance with the manufacturer's drawings. The Special Posts on Sheet 24, including Special Steel Posts, Encased Posts, and Frangible Leave-Outs, are not permitted within the Approach Terminal segment unless otherwise called for in the plans.

Align panel lap splices in accordance with the manufacturer's drawings,

Install adjacent grading, gutters, and/or curbing as shown herein.

2. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments.

Approach Transitions, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown

3. APPROACH TERMINAL TEST LEVEL: Install either a Test Level 3 (TL-3) or Test Level 2 (TL-2) Approach Terminal as specified in the plans. TL-3 Approach Terminals may substitute for TL-2 Approach Terminals unless the substitution is specifically prohibited in the plans. TL-2 Approach Terminals may not substitute for TL-3 installations.

4. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification 536.

5. 2" MISCELLANEOUS ASPHALT PAVEMENT: The Plan View depicts the Unpaved Shoulder condition. For Fully Paved Shoulder and Shoulder Gutter conditions, extend the 2" Misc. Asphalt Pavement as shown in the

The 2" Misc. Asphalt Pavement shown upstream of Post (1) may be substituted with a different pavement type where called for in the Plans.

6. CLEAR AREA REQUIREMENT: Do not place any permanent aboveground installations within the areas shown with 1:10 maximum grading. For the finished condition, keep this area free of all aboveground obstructions,

7. 'CURBED' AND 'DOUBLE FACED' GUARDRAIL SEGMENTS: See Sheet 8.

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NOTES:

- 1. GENERAL: See Notes 1 through 3 on Sheet 7.
- 2. CURBED SEGMENTS: Type E curb is required within the limits shown. When a different curb type is called for outside of the Type E curb limits, transition the curb shape linearly, over a nominal distance ranging 5'-0" to 10'-0"
- 3. TAPER LENGTH: For Curbed Segments, taper the guardrail away from the roadway where shown to place the inside edge of the Impact Head at 5" behind the face of the curb. Where additional lateral offset is required to fit the Approach Terminal Assembly hardware, such as a soil plate, place the Impact Head as close to the curb as the hardware allows, not to exceed 2'-0" from the face of curb.
- 4. GUARDRAIL HEIGHT TAPER: For Curbed Segments, the connecting General Guardrail Mounting Height, 'H', is typically measured from the Lip of Gutter (See Sheet 6 Guardrail Sections, 'Adjacent to Curb'), while the End Terminal Assembly 'H' is measured from the Misc. Asphalt Pavt. (See Section A-A). Linearly taper the difference in Mounting Height over a minimum length of 12'-6", starting where indicated herein.
- 5. DOUBLE FACED SEGMENT: Connect to Double Faced General Guardrail. Use consistent Posts and Offset Block types as specified in the APL drawings over the entire Length of End Treatment, 'LE'. Posts and Offset Blocks in the adjoining General Guardrail segment may be different from those inside of the 'LE'. A change in post type between timber and steel is permitted, immediately outside of the 'LE' segment.

Maintain the 1:10 maximum grading as shown in Section B-B throughout segment 'LE'. Where required, transition to differing adjacent slopes linearly, over a minimum longitudinal length of 25'-0".

- 6. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification 536.
- 7. CLEAR AREA REQUIREMENT: Do not place any permanent aboveground installations within the areas shown with 1:10 maximum grading. For the finished condition, keep this area free of all aboveground obstructions, including dense vegetation and trees.
- 8. 2" MISCELLANEOUS ASPHALT PAVEMENT: The 2" Misc. Asphalt Pavement shown upstream of Post (1) may be substituted with a different pavement type where called for in the Plans.
- 9. SINGLE FACED 'PARALLEL' SEGMENTS: See Sheet 7.

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NOTES:

1. INSTALLATION: Construct the Approach Transition segment where indicated in the plans. For example Layouts showing the Approach Transition's fit among other guardrail segments, see Sheet 19.

For existing bridge connection options, see Indexes 536-002, 521-404, and 521-405.

- 2. SECTION VIEWS & DETAILS: For cross sections and details, including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 17.
- 3. GUARDRAIL TAPER: The connecting guardrail may require a different lateral offset if shown in the plans. At the location indicated herein, taper the guardrail to the connecting guardrail offset. If the adjacent guardrail segment has the same offset as the Approach Transition segment, then no taper is required.
- 4. END TRANSITION OF CURB OPTIONS: The Plan and Elevation views depict an example Curb Transition to Flush Shoulder from Section D-D to E-E, but this transition may require a different shape depending on the End Transition option shown in the plans (Either a 'Shoulder Gutter Option', 'Raised Curb Option', or 'Flush Shoulder Option'). See Sheet 16 for additional curb options and Sheet 17 for curb shape details.
- 5. RIGID BARRIER END TRANSITION: Taper the Rigid Barrier toe as shown. See Concrete Barrier, Index 521–001, and Traffic Railing, Indexes 521–422 and 521–428, for details.
- 6. OFFSET: The required offset difference between the Face of Guardrail and Rigid Barrier Shoulder Line is considered negligible and may not be shown in the guardrail offset callouts in the plans. A consistent guardrail offset deviation of up to 4 inches outside of the Rigid Barrier Shoulder Line is permitted over the length 'LA'.
- 7. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments. Approach Terminals, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.

APPROACH TRANSITION CONNECTION TO RIGID BARRIER - LOW-SPEED, TL-2

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WITH 'SHOULDER GUTTER' CONNECTION - PLAN VIEW



NOTES:

- 1. GENERAL: See the applicable notes and details on Sheet 15.
- including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 17.
- 3. ELEVATION VIEW: For post and panel installation details within 'LA', see the elevation view on Sheet 15. The curb details will differ depending on curb option required.

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NOTES:

- 1. GENERAL: See the applicable notes and details on Sheet 15.
- 2. SECTION VIEWS AND DETAILS: For cross sections and details, including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 17.
- 3. RIGID BARRIER CONNECTION: For additional connection details, see Sheet 20.

TRAILING END TRANSITION CONNECTION TO RIGID BARRIER CURR CONNECTIONS

- COND CONNECTIONS		
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- and connections where called for in the plans.
- Index, refer to Index 536-001.

NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- curb blunt ends are not in place.
- Specification 967.

GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT

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FY 2024-25 STANDARD PLANS

GUARDRAIL TRANSITIONS CONNECTIONS FOR EXISTING

GENERAL NOTES

1. This index provides guardrail transition details for approach and trailing end guardrail connections to existing bridges, including details for connecting to traffic railing retrofits and safety shape barriers on existing bridges. Sheets 1 through 26 apply to bridges with retrofitted traffic railings (Sheet 26 shows the trailing end guardrail connections). Sheets 27 and 28 apply to bridges with safety shape traffic railing, and they provide approach and trailing end transition connection details for guardrail. Construct these guardrail transitions

2. For miscellaneous guardrail components and construction details that are not provided in this

1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.

2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to

3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated in accordance with

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification 937 and be installed in accordance with Specification 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 15 through 18 and the flag notation on Sheet 26.

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CONNECTIONS FOR EXISTING BRIDGES



Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

GUARDRAIL LENGTHS	
MEDIAN 6' BRIDGE SHOULDERS 10' BRIDGE SHOULDERS	
WIDTH 1:10 TAPER RATE 1:15 TAPER RATE 1:10 TAPER RATE 1:15 TAPER	RATE
(Ft.) PANELS (No.) LENGTH (Ft.) PANELS (No.) LENGTH (Ft.) PANELS (No.) LENGTH (Ft.) PANELS (No.) LE	LENGTH (Ft.)
<u>30</u> <u>12.5</u> <u>156.25</u> <u>18.5</u> <u>231.25</u> <u>6.5</u> <u>81.25</u> <u>9.5</u>	118.75
28 11.5 143.75 16.5 206.25 5.5 68.75 7.5	93.75
26 9.5 118.75 14.5 181.25 5.5* 68.75 5.5*	68.75
24 8.5 106.25 11.5 143.75 5.5* 68.75 5.5*	68.75

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see * below.

*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH CONCRETE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS





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SEE INDEXES 460-473 & 460-476 - SCHEMES 5 & 6

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

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GUARDRAIL TRANSITION CONNECTIONS FOR EXISTIN

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PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 521-482 SHOWN, INDEX 521-405 SIMILAR)



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/17/2023	TRAILING END GUAR	DRAIL AND ANCHORAGE FOR	BRIDGE TRAFFIC RAILING (THRIE BEAM RETROFITS)
10	LAST DESCRIPTION: REVISION 11/01/19	FDOT FY 2024-25 STANDARD PLANS	GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES





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GUARDRAIL TRANSITION CONNECTIONS FOR EXISTING

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GENERAL NOTES:

- 1. GENERAL: Work this Index in accordance with Specification 544 and the "Summary of Permanent Crash Cushions" table in the Plans. Where applicable, use Guardrail components and panel lap splices in accordance with Index 536-001.
- 2. TRANSITION PANEL: Where crash cushions are placed between two-way traffic or adjacent to two-way two-lane traffic, place a Transition Panel from the Concrete Barrier to the Crash Cushion on the downstream side of the barrier end (as shown). Follow the requirements of the APL drawing.
- 3. MANUFACTURER'S TRANSITION: Construct the proprietary guardrail transition only if shown in the applicable APL drawing. See Note 4 below.
- 4. STANDARD GUARDRAIL TRANSITION: If the APL drawing does not provide a guardrail transition to w-beam guardrail, construct the Standard Guardrail Transition segment from thrie-beam to w-beam as shown per Sheet 2. This 21'-10¹/₂" segment must remain parallel to the roadway.

If the APL drawing does provide a guardrail transition to w-beam guardrail, replace the Standard Guardrail Transition segment with a w-beam guardrail segment at 6'-3" post spacing, except that Post (10) will remain where shown herein if it is located at a guardrail begin or end taper station callout per the Plans. This 21'-10¹/₂" segment must also remain parallel to the roadway.

5. LENGTH OF END TREATMENT: For Crash Cushions, the Length of End Treatment includes all proprietary elements of the design as shown in the APL drawing, including the manufacturer's transition of guardrail if applicable.

The actual Length of End Treatment varies per Crash Cushion type, but an estimated Length of End Treatment is generally shown in the Plans to provide sufficient space for the Contractor's option of differing Crash Cushion types.

- 6. LENGTH RESTRICTION: In the "Summary of Permanent Crash Cushions" table, if a value is provided in the Length Restriction column, then select a Crash Cushion from the APL which has a Length of End Treatment less than or equal to the value shown. If the table instead shows not applicable (N/A), then Crash Cushion selection is unrestricted regarding length.
- 7. CRASH CUSHION STATION: The Crash Cushion Station point shown herein corresponds to the station provided in the "Summary of Permanent Crash Cushions" table in the Plans.



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nd Concrete Barrier or Traffic Railing Sta.		
ishion Sta. (See Note 7)		
Concrete Barrier (See Index 521-001)	-	
nsition Panel (See Note 2)		
<u> </u>		
Concrete Barrier or Traffic Railing (Double Faced Shown, Single Faced Similar)	I	
PPLICATION		
Begin/End Guardrail Sta. (@ Post)		
Crash Cushion Sta. (See Note 7)		
Standard Guardrail Transition Guardrail (If Applicable, See Note 4) (See Index 536)	5-001)	
(1) (2) (3) (10)	1	
Guardrail (Double Faced Shown, Single Faced Similar)		
_ -		
/// /ON		
PERMANENT CRASH CUSHION	APPLICA	TIONS
ON DETAILS	INDEX	sheet 1 of 3





Cushion Sta. (See General Note 7)
/End Guardrail Sta. (@ Post)
Length of End Treatment
(See General Note 5)
acturer's Transition
ole, See General Note 3)
(
· · · · ·

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TABLE 1 - BRAKING ZONE			
Posted	111		
Speed	(Foot)		
(mph)	(reel)		
≤ 30	150		
35	200		
40	250		
45	300		
50	350		
55	410		
60	470		
65	550		

- 1. Construct permanent raised rumble strips where shown in the Plans and in accordance
- 2. Use color white for preformed thermoplastic

PERMANENT RAISED	RUMBL	E STRIPS
IPS	INDEX	SHEET
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OPTION ST1 - ASPHALT SET



OPTION ST2 - PREFORMED THERMOPLASTIC SET



OPTION ST3 - REMOVABLE POLYMER STRIPING TAPE SET

_____ OPTIONAL MATERIALS DETAILS _____



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NOTES:

- 1. Construct short-term raised rumble strips where noted in the Plans and in accordance with Specification 546.
- 2. See Sheet 1 for placement and additional details.
- 3. Use color white for Preformed Thermoplastic and Removable Polymer Striping Tape Sets.

SHORT-TERM RAISED RUMBLE STRIPS

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NOTES:

- 1. When friction course extends more than 8" beyond the edge of the traveled way, blade off the extended friction course to the 8" line prior to rumble strip grinding.
- 2. Use the continuous array on both inside and outside shoulders 1,000 feet in advance of bridge ends or back to the gore recovery area for mainline interchange bridges. Use the skip array for all other locations.
- 3. Exclude rumble strips at the following locations:
- A. At mainline tolling areas, terminate rumble strips at the end of the mainline normal section.
- B. At All Electronic Tolling (AET) facilities, terminate rumble strips within 50 feet of the centerline of the overhead gantry.
- C. On outside shoulders of entrance ramp terminals, terminate rumble strips at the point of the physical gore and resume at the end of the acceleration lane taper.
- D. On outside shoulders of exit ramp terminals, terminate rumble strips at the start of the deceleration lane taper and resume at the point of the physical gore.
- E. On approaches to bridges, terminate rumble strips at the approach slab joint.







T,	TABLE 2						
LE	STRIP DEPTH						
oth ī	th Tolerance = $\pm \frac{1}{16}$)						
v	I DEPTH FROM SURFACE (IN.)						
1/16							
%6							

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NOTES

DESIGN CRITERIA:

1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and Chapter 3 of the FDOT Structures Design Guidelines.

SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

MATERIALS:

1. See Specification Section 548 for material requirements.

CONSTRUCTION:

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. If required, locate manholes and drop inlets as shown on wall elevations.
- 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 8. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.
- 9. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 10. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 11. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 12. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- 13. The top of the leveling pad or footing will be 2'-0" minimum below final ground line.
- 14. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 15. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 16. Work this Index with Index 521-600 thru 521-650.

SHOP DRAWINGS:

See Specification Section 548 for shop drawing requirements.

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MSE RETAINING WALL SYSTEMS

Concrete Barrier

Top of Coping Elevation

Concrete Coping -



ELEVATION VIEW OF COPING HEIGHT TRANSITION

	FDOT MSE RETAINING WALL CLASSIFICATION TABLE												
Applicable	Durability Requirements (Carbon-Steel Reinforcing)			Durability Requirements (FRP Reinforcing)		Soil	0the	r Allo	wable	FD0T	Wall	Types	
FDOT Wall	Concrete	Concrete	Pozzolan	Concrete	Concrete	Pozzolan	Reinforcement						
Type *	Cover	Class	Additions?	Cover	Class	Additions?	Туре	2A	2B	2C	2D	2E	2F
	(in.)	for Panels	**	(in.)	for Panels	**							
Type 2A	2	II	No	1.5	II	No	Metal		1	1	1	1	1
Type 2B	2	IV	No	1.5	IV	No	Metal			1	1	1	~
Type 2C	3	IV	No	1.5	IV	No	Metal				1	1	~
Type 2D	3	IV	Yes	2	IV	No	Metal						~
Type 2E	3	IV	No	2	IV	No	Plastic						-
Type 2F	3	IV	Yes	2	IV	No	Plastic						

* See Data Table in Contract Plans.

** Highly Reactive Pozzolans.





DESCRIPTION:



TYPICAL MSE RETAINING WALL SECTION WITH A CONCRETE BARRIER (Showing Limits of the Reinforced Soil Volume)

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NOTES

DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and FDOT Structures Design Guidelines Section 3.13.2.
- 2. It is the responsibility of the Engineer to determine that the factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer.
- 4. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.

SOIL PARAMETERS:

1. See wall control drawings for soil characteristics of foundation material to be used in the design of the wall system. The Contractor must provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site. Provide the values of unit weight, cohesion and internal friction angle in the Shop Drawings.

MATERIALS:

- 1. Provide soil reinforcement in accordance with Specification Section 548.
- 2. For additional material notes, see Wall Company General Notes.

CONSTRUCTION:

- 1. Walls must be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. Refer to Plan and Elevation sheets of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 4. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor must notify the Engineer to determine what course of action should be taken.
- 5. The Contractor is responsible for gradually deflecting upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.



TYPICAL RETAINING WALL SECTION (Showing Limits of the Reinforced Soil Volume)



PLACEMENT DETAIL

	NΟ	DESCRIPTION:
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FY 2024-25 STANDARD PLANS

MSE RETAINING WALL SYSTEMS

Temporary Barrier (See Index 102-100)

GENERAL NOTES AND DETAILS

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GENERAL NOTES:

- 1. This fence to be provided generally in rural areas. For supplemental information see Specifications 550.
- 2. Fabric shall be woven wire, either galvanized steel, meeting the requirements of ASTM A116, No. 9 Grade 60, Design Number 1047-6-9, with Class 3 zinc coating; No. 12 ½ Grade 175, Design Number 1047-6-12 ½, with a 10 ½ gage top and bottom wire and with Class 3 zinc coating; or aluminum coated steel, meeting the requirements of ASTM A584, No. 9 Farm, Design Number 1047-6-9, with a minimum coating weight of 0.40 oz./ft.?. For additional information see payment note below.
- 3. Fence shall be installed with wire side to private property except on horizontal curves greater than 3° the fence shall be installed so as to pull against all posts.
- 4. Posts may be either timber, steel, recycled plastic or concrete. Unless a specific post material is called for in the plans, the Contractor may elect to use either a single material or a combination of timber, steel, recycled plastic or concrete materials, but must comply with the electrical grounding requirements in Specification 550. Line posts of one material may be used with corner, pull and end post assemblies of a different material. Line posts of only one optional material and pull post assemblies of only one optional material will be permitted between corner and end post assemblies. Within individual corner and end post assemblies only one optional material will be permitted.
- 5. Timber posts shall meet the material requirements of Specification 954. Timber line posts are to be minimum 4" diameter. Timber corner, pull, approach and end posts are to be a minimum 5" diameter. Timber braces are to be minimum 4" diameter.
 - (A) Staples for line posts to be $1\frac{1}{4}$ " minimum length; for approach, corner and pull posts $1\frac{1}{2}$ " minimum length. At approach, corner and pull posts, staple every line wire. At line posts, staple every line wire in top half and alternate line wires in bottom half. Staples shall be driven diagonally across the line wire with the points in separate grains.
 - (B) Connections between timber posts and braces to be provided by dowels as shown in fastener details. (C) Wire to be wrapped and tied, as shown in the splice details, at the following locations:
 - (a) All end posts, (b) Corner post, including the assemblies at vertical breaks of 15° or more and (c) Pull posts where the wire is not spliced and pulled through the assembly; see General Note 18.
- 6. Steel posts and braces shall be standard steel posts, galvanized at the rate of 2 oz./ft.², together with necessary hardware and wire clamps and meeting the following requirements:
 - (A) Line posts: 8' long; 1.33 lbs./ft.; hot rolled studded; anchor plate attached, ASTM A702 (18 in.²). (B) Approach posts: $2\frac{1}{2}$ "x $2\frac{1}{2}$ "x $\frac{1}{4}$ " angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - (C) Pull, end and corner posts: $2\frac{1}{2}x\frac{1}{4}x\frac{1}{4}$ angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - (D) Braces: $2''x2''x''_4''$ angles with necessary hardware and fabricated for attaching to post.
 - (E) The pull, corner, approach and end posts are to be set in concrete as per detail. (Also see General Note(15)
- 7. Recycled plastic posts shall meet the following material requirements: Line posts shall have a minimum section of 4" round or 4" square. Plastic posts shall not be used as corner, pull, end or approach posts unless such use is specifically detailed in the plans. The straightness of the post shall comply with Specification 954 for timber post. The flexural strength shall meet the requirements of the latest edition of the Southern Pine Inspection Bureau's Standard Grading Rules for Southern Pine Lumber for No. 2SR Stress Rated Grade Timber. Plastic posts can be set by either digging and tamped backfill or by driving into full depth preformed holes 1/4" to 1/2" smaller than cross section of

Staples for fabric and barbed wire connection to plastic line posts shall be the same size, count and location as that for timber posts.

- 8. The Contractor, at his option, may use any suitable precast or prestressed concrete posts; however, approval by the Engineer, of posts not shown on this index, will be required prior to construction of the fence. Precast posts shall be Class II concrete. Prestressed posts shall be Class III concrete. Lengths of concrete post to be as indicated for timber posts.
- 9. Aluminum post, braces and accessory framing hardware shall not be used unless the plans specifically detail their application or the Engineer specifically approves their incorporation in fence construction or repair. Aluminum framed gates are permitted as described in General Note 19.

- 10. The woven wire shall be attached to steel and concrete posts by a minimum of five tie wires. The single wire ties shall be applied to the top, bottom and three intermittent line wires. The ends of each tie wire shall have a minimum of two tight turns around the line wire. Tie wires shall be steel wire not less than 0.120" diameter, zinc coating Class 3, soft temper, in accordance with ASTM A641.
- 11. Steel Barbed Wire can be either of the following types:

Type I: This type shall conform to the requirements of ASTM A121, with two strands of 12¹/₂ gage wire; four-point barbs, wire size 14 gage, twisted around both line wires; and, Class 3 coating, Design No. 12-4-5-14R.

- Type IIA: This type same as Type I except the two strand wires are twisted in alternating directions between consecutive barbs
- Type IIB: This type shall conform to the requirements of ASTM A121 with two strands of 15 ½ gage high tensile Design No. 15-4-5-16R.

Aluminum Barbed Wire shall be fabricated of two strands of 0.110-inch wire with 0.08-inch diameter four-point barbs spaced at approximately $5\frac{1}{2}$ ", and at a maximum spacing of 6". The wire for the strands and for the barbs shall be of ASTM B211M Alloy 5052-H38 or equal.

- 12. The woven wire shall be stretched only until one-half the tension curl has been pulled out of the line wires.
- 13. Posts to be set by driving or digging. If by digging, the posts shall be set at the center of the hole and the soil tamped securely on all sides.
- 14. Longer posts than those indicated above may be required by the plans or for deeper installations.
- 15. Concrete bases for angular steel posts (pull, corner, end and approach) shall be Class NS in accordance with Specification 347. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 16. Pull post assemblies shall be installed at approximately 330' centers except that this maximum interval may be reduced by the Engineer on curves where the radius is less than 3°.
- 17. Corner post assemblies are to be installed at all horizontal and vertical breaks in fence of 15° or more.
- 18. A maximum length of 1320' of wire may be installed as a unit. For pulls through a pull post assembly the fabric shall be spliced by crimping sleeves only. Pulls through a corner post assembly will not be permitted.
- 19. Unless otherwise called for in the plans gates shall be commercially available metal swing gates assembled and installed in accordance with the manufacturer's specifications as approved by the Engineer. Chain link swing gates in accordance with Index 550-002 may be substituted for metal swing gates as approved by the Engineer. Gate size is full opening width whether single leaf or double leaves. Payment for gates shall include the gate, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 20. For construction purposes, assemblies are defined as follows: End post assemblies shall consist of: one end post, one approach post, two braces, four diagonal tension wires and all necessary fittings and hardware. Pull post assemblies shall consist of: one pull post, two braces, four diagonal tension wires and necessary fittings and hardware. Corner post assemblies shall consist of: one corner post, two approach posts, four braces, eight diagonal tension wires and all necessary fittings and hardware.
- 21. All posts, braces, tension wires, fabric, tie wires, Class NS concrete, and all miscellaneous fittings and hardware to be included in the cost for Fencing, LF. Fencing shall be inclusive of the lengths of pull, end and corner post assemblies, but exclusive of gate widths.



wire; four-point barbs, wire size 16 $\frac{1}{2}$ gage twisted around both line wires; and Class 3 coating,

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GENERAL NOTES:

- 1. This fence to be used generally in urban areas.
- 2. For supplemental information refer to Specification 550.
- Chain link fabric, post, truss rods, tension wires, tie wires, stretcher bars, gates and all miscellaneous З. fittings and hardware shall meet the requirements of AASHTO and ASTM signify current reference.
- 4. Fence Component Options:
 - A. Line post options:
 - (1) Galvanized steel pipe, Schedule 40- 1½ nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table 2 (Grade A or B), ASTM F1083, and AASHTO M111.
 - (2) Aluminum coated steel pipe: ASTM A53, Table 2 (Grade A or B): Schedule 40- 1¹/₂" nominal dia., 1.90" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
 (3) Aluminum alloy pipe- 2" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.

 - (4) Steel H-Beam- 1⁷/₈"x 1⁵/₈": Zinc Galv. 1.8 oz./ft.: AASHTO M111 and Detail.
 - (5) Aluminum alloy H-Beam- 17/8"X 15/8" Detail
 - (6) Steel C- 1⁷/₈"X 1⁵/₈": Galv.: 1.8 oz/ft. zinc: AASHTO M111; OR , 0.9 oz./ft². zinc-5% aluminum-mischmetal: ASTM F1043 and Detail.
 - (7) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 2" 0D, 1¹/₂" NPS, 1.900" dec. equiv., 0.120" min. wall thick. and min. wt. 2.28 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of $15\mu g/in^2$. min, and the polymer film topcoat shall have a thickness of 0.0003° min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.

 - B. Corner, end, and pull post options: (1) Galvanized steel pipe, Schedule 40- 2" nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.
 - (2) Aluminum coated steel pipe: ASTM A53 steel, X 2 Tables: Schedule 40; 2" nominal dia., 2.375" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.

 - (3) Aluminum alloy pipe- 2¹/₂" nominal dia.: ASTM B241 or B221, Alloy 6063,T6.
 (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 2¹/₂" OD, 2" NPS, 2.375" dec. equiv., 0.130"min. wall thick. and min. wt. 3.117 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15µg/in². min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.
 - C Rail options
 - (1) Galvanized steel pipe, Schedule 40- 11#4" nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.
 - (2) Aluminum coated steel pipe; ASTM A53 steel, X 2 Tables Schedule 40; 11#4" nominal dia., 1.660" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
 - (3) Aluminum alloy pipe- 11#4" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
 - (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 15#8" OD, 11#4" NPS, 1.660" dec. equiv., 0.11'1" min. wall thick. and min. wt. 1.836 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15µg/in². min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.
 - D. Chain link fabric options (2" mesh with twisted and barbed selvage top and bottom for all options except as described in Note 10):
 - (1) AASHTO M181 Type I Zinc Coated Steel, No. 9 gage (coated wire diameter), coated at the rate of 1.8 oz/ft². (M181 Class D 2.0 oz./ft². modified to 1.8 oz./ft².).
 (2) AASHTO M181 Type II -Aluminum Coated Steel, No. 9 gage (coated wire diameter), coated
 - at the rate of 0.40 oz./ft².
 - (3) AASHTO M181 Type IV- Polyvinyl Chloride (PVC) Coated Steel, No. 9 gage (coated core wire diameter), core wire-zinc coated steel. PVC coating: M181 Class A (either extruded or extruded and bonded) or Class B (bonded). See table right. Unless the plans call for M181 standard colors medium green, dark green or black the coating color shall be soft gray matching that of No. 36622 of Federal Standard 595a.
 - E. Tension wire options.
 - (1) Steel wire No. 7 gage zinc galvanized at the rate of 1.2 oz./ft².: AASHTO M181. (2) Aluminum alloy wire with a diameter of 0.1875" or larger conforming to the requirements of ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
 - (3) Aluminum coated steel wire No.7 gage coated at the rate of 0.040 oz./ft².: AASHTO M181.

 - F. Tie wire and hog ring options:
 (1) Steel wire No.9 gage zinc galvanized at the rate of 1.2 oz./ft².
 (2) Aluminum alloy wire with a diameter of 0.1443" or larger conforming to the requirements of ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
 (3) Aluminum coated steel wire No. 7 gage coated at the rate of 0.040 oz./ft².

Post Cap Ties @ 2' Centers Ties @ 12" Centers б. St Chain Link Fabric No. 9 Gage õ 2" Mesh, Twisted And Barbed Top And Bottom Selvage — Ties @ 2' Centers 🔨 _ 10 LINE POST LINE POST









DESCRIPTION:



GENERAL NOTES CONTINUED

- 5. Unless a specific material is called for in the plans the Contractor may elect to use either a single type of material or a combination of material types from the component options listed in note 4. Combinations of optional materials are restricted as follows:
 - (a) Only one fabric optional material will be permitted between corner and/or end post assemblies. (b) Only one line post optional material will be permitted between corner and/or end post assemblies.
 - (c) Pull post assemblies shall be optional materials identical to either the line post optional material or the corner and end post assembly optional material; but, pull post assemblies shall be the same optional material between any set of corner and/or end post assemblies.
- 6. Concrete for bases shall be Class NS concrete as specified in Specification 347 or a packaged, dry material meeting the requirements of a concrete under ASTM C-387. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 7. Line post shall be 8'-6" long (Standard). Line post are to be set in concrete as described above or by the following methods:
 - (a) In accordance with special details and/or as specifically described in the Contract Plans and Specifications.
 - (b) In accordance with ASTM F567 Subsections 5.4 through 5.10 as approved by the Engineer. Line post installed in accordance with Section 5.8 shall be 9'-6" long.
 - (c) Post mounted on concrete structure or solid rock shall be mounted in accordance with the base plate detail "Fence Mounting On Concrete Endwalls And Retaining Wall", Sheet 3; or, by embedment in accordance with ASTM F567 Subsection 5.5.

End, pull and corner post assemblies shall be in concrete as detailed above for all soil conditions other than solid rock. Post within assemblies that are located on concrete structures or solid rock shall be set by base plate or by embedment as prescribed under (b) above for line post.

Line and assembly posts for 6' fence which must be lengthened due to a variation in the normal ground clearance, shall be set an additional 3" in depth for each 1' of of additional ground clearance.

- 8. Pull post shall be used at breaks in vertical grades of 15° or more, or at approximately 350' centers except that this maximum interval may be reduced by the Engineer on curve's where the curve is greater than 3°.
- 9. Corner post are to be installed at all horizontal breaks in fence at 15° or more and as required at vertical breaks over 15° as determined by the Engineer.
- 10. When fence has an installed top of fabric height less than 6' knuckled top and bottom selvages shall be used unless the plans specifically identify locations for twisted selvage fabrics.
- 11. Unless sliding gates or special gates are called for in the plans, all gates shall be chain link swing gates meeting the material requirements described and as approved by the Engineer. Payment shall include the gates, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 12. For construction purposes corner post assemblies shall consist of one corner post, two braces, two truss rods, and all necessary fittings and hardware as detailed. End post assemblies shall consist of one end post, one brace, one truss rod and all necessary fittings and hardware as detailed.
- 13. In areas where there are physical constraints outside the right-of-way which restricts the fence construction, the fabric may be installed on the inside of the posts ...

	TYPE IV VINYL COATED FABRIC								
	AASHTO M181 Table 4 Redefined As Follows								
PVC Thickness Range									
<i>Specified Diar Of Metallic Co Core Wire</i>		meter oated	Minimum Weight Of Zinc Coating		M181 Class A (Extruded Or Extruded And Bonded Coating)		M181 (Bonded	Class B Coating)	
in.	mm	gage	oz./ft².	g/m²	in.	mm	in.	mm	
0.148	3.77	9	0.30	92	0.015 to 0.025	0.38 to 0.64	0.006 to 0.010	0.15 to 0.25	

This index details fencing that is constructed with chain link fabric 6' (nominal) in height and with specific ground clearance. For fencing of different height or installation details, the fence shall be fully detailed in the Contract plans.

	2	DESCRIPTION:
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-	17	



DESIGN NOTE

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APPLIES TO BRIDGE OVER CROSSROAD AND CROSSROAD OVER FREEWAY (BRIDGE OVER CROSSROAD SHOWN)

FENCING TERMINALS AT RURAL INTERCHANGES





FENCING TERMINALS AT URBAN INTERCHANGES



DESCRIPTION:



FENCE LOCATION

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WILDFLOWER SEEDING RATES				
Common Name (Botanical Name)	lbs/a			
#1 Group				
Black-Eyed Susan (Rudbeckia hirta)	2			
Lance-Leaf Tickseed (Coreopsis lanceolata)	10			
Goldenmane Tickseed (Coreopsis basalis)	10			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Softhair Coneflower (Rudbeckia mollis)	2			
Crimson Clover (Trifolium incarnatum)	15			
#2 Group				
Annual Phlox (Phlox drummondii)	10			
Moss Verbena (Verbena tenuisecta)	6			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Crimson Clover (Trifolium incarnatum)	15			
Note: Wildflower seeding rates are for restoring impacted wildflower areas.				



GENERAL NOTES

1. All turf establishment shall be performed meeting the requirements of Specification 570.

- 2. Activities such as clearing, grading, and excavating that will disturb one or more acres of land require coverage under the Generic Permit for Stormwater Discharge from Large and Small Construction Activities from the Florida Department of Environmental Protection, and implementation of appropriate pollution prevention measures to minimize erosion and sedimentation and properly manage stormwater.
- 3. Confirm compatibility of wildflower with Seeding Zones.







FY 2024-25 STANDARD PLANS

PERMANENT EROSION CO





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	- .	
		_
5' Shoulder Pavement — See Table Below		~
For Application	$\frac{1}{80'}$ $\int_{20'}^{-}$ $\frac{1}{20'}$	
UNDIVIDED		
Shoulder Pavem <u>ent</u>		
4' Shoulder Pavement-See	e Table Below_F	or Application.
For Facilities With Unpave	ed Shoulders.	
Shoulder Pavement		
	300'	
	*	
DIVIDED 4/		
Shoulder	Point	
ON DIVIDED AND UNDIVIDED FACILIT	IES	
Note:		
Shoulder Pavement is required on al meeting the criteria tabulated. For a	l curves	
not meeting the criteria, shoulders a	re to	
be paved where erosion of the shoul evident or anticipated	der is	
NT FOR SUPERELEVAT	ED ROA	DWAYS
nents are applicable to new constructio oject requirements for shoulder pavem this standard take precedence.	n, reconstructic ent and sodding	on and RRR g that exceed
adjacent to ditches and at headwalls,	see Index 524-	001.
pes steeper than 1:3 are to be sodded.		
ROSION AND SHOULD	DER RAVI	ELING
	INDFX	SHFFT
NTROI	TINDEN	SILLI

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																					_
						TABLE 2: SC	DD (วบิA	NTI	TIES	5 (5	Y)									
	INDEX 430-010	INL	DEX 4	30-011		INDEX 430-020	INDEX 430-030						INDEX 43								
		SLOPE				SLOPE SLOPE						SLC	SLOP								
PIPE SIZE	1:4	1:2	1:3	1:4	1:6	1:2			1:3 1:4				1:6			1:2	1:3				
			PIPE	5		PIPES	PIPES						PIPE								
	1	1	1	1	1	1	1	2	3	1	2	3	1	2	3	1	2	3	1	1	
12"						10													14	15	
15"	15	13 (15)	16	17	23	11	19	21	24	22	26	29	26	30	33	34	38	43	15	17	
18"	16	14 (16)	17	19	25	11	21	24	27	25	29	33	30	34	38	39	44	50	16	18	
21"						12															
24"	19	15 (17)	19	21	28	14	26	30	34	32	37	42	38	44	50	50	58	66	19	22	
27"						15															
30"	21	17 (18)	21	24	32	16	31	37	42	39	46	53	46	55	63	62	74	85	21	25	
36"						18	37	44	52	46	56	65	56	67	79	76	91	107	24	29	
42"						19	43	53	62	55	67	79	67	82	96	91	111	132	27	32	
48"						21	50	62	73	64	79	93	78	97	115	108	133	158	30	36	
54"						21	57	71	85	74	92	110	91	113	136	126	157	188			
60"						22															
66"						25															
72"						26															
		() Endw	all Wi	th Bai	ffles																











STANDARD PLANS

TURF ON EXISTING FACILITIES







11/01/19

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