

FY 2024-25 STANDARD PLANS FOR BRIDGE CONSTRUCTION

Effective for Projects with Lettings in the Fiscal Year (FY) from

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State of Florida Department of Transportation
Office of Design
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FY 2024-25 Standard Plans for Road and Bridge Construction Topic No. 625-010-003

ABBREVIATIONS

FY 2024-25 STANDARD PLANS

Abbreviation	Meaning	Abbreviation	4-25 STANDARD PLANS Meaning	Abbreviation	Meaning
	4 	(S 		G
AASHT0	American Association Of State Highway And Transportation Officials	CSIP	Cost Savings Initiative Proposal	G	Shear Modulus
AC	Alternating Current	CSL	Cross-hole Sonic Logging	g	Gram
Accel.	Acceleration	СТРВ	Cement Treated Permeable Base	Ga.	Gauge or Gage
ACI	American Concrete Institute	Ctr., Ctrs.	Center	Galv.	Galvanized
ADA	Americans With Disabilities Act	Cu. Ft.	Cubic Feet	GFI	Ground Fault Interrupter
ADT	Average Daily Traffic	Cu. Yd., CY,	Cubic Yard	GFRP	Glass Fiber Reinforced Polymer
AFAD	Automated Flagger Assistance Device	[D	Grd.	Ground
AISC	American Institute Of Steel Construction	D	Depth, Distance or Diameter		H
AISI	American Iron and Steel Institute	Dia. or Ø	Diameter	Hd.	Head
Alt.	Alternate	Dbl.	Double	H.S., HS	High Strength
Alum.	Aluminum	Decel.	Deceleration	HDPE	High Density Polyethylene
ANSI	American National Standards Institute	Deg.	Degree	Horiz.	Horizontal
A05	Apparent Opening Size	Dim.	Dimension	HP	Horsepower or H-Pile
APL	Approved Products List	Dist.	Distance	HSHV	High Strength Horizontal Vertical
Арр.	Approach	DMM	Domestic Mail Manual		I
Approx.	Approximate	DPI	Ditch Point Intersection	ID, I.D.	Inside Diameter or Identification
ARTBA	American Road & Transportation Builders Association	Dt	Ditch	in.	Inch(es)
Asph.	Asphalt	DT0E	District Traffic Operations Engineer	Inc.	Incorporated
Assem.	Assembly	I		Int.	Interior
ASTM	American Society For Testing And Materials	е	Superelevation Rate	Inv.	Invert
ATPB	Asphalt Treated Permeable Base	E.P. or EOP	Edge Of Pavement	ITS	Intelligent Transportation Systems
Auxil.	Auxiliary	EA or Ea.	Each		J
AWG	American Wire Gauge	EIA	Electronic Industries Alliance	JCT	Junction
AWS	American Welding Society	El. or Elev.	Elevation	Jt.	Joint
L	B	Embed.	Embedment		Κ
Bot.	Bottom	EPDM	Ethylene Propylene Diene Monomer	k	kip
Brkwy.	Breakaway	Eq.	Equation or Equal	kip	1000 Pounds
b/w	Between	Equip.	Equipment	ksi	Kips Per Square Inch
(9	etc.	Et Cetera (And So Forth)	kVA	Kilovolt Ampere
CC, C to C	Center to Center	ETP	Electronic Tough Pitch		L
C & G	Curb And Gutter	Ex.	Example	L	Length
C.C.	Crash Cushion	Exist.	Existing	LA	Limited Access
CCTV	Closed-Circuit Television	Exp.	Expansion	lb or lbs.	Pound(s)
CFR	Code of Federal Regulations	Ext.	Extension	lb/sy	Pounds Per Square Yard
CFRP	Carbon Fiber Reinforced Polymer	I	F	lbf	Pound force
cfs, CFS	Cubic Feet Per Second	FAC	Florida Administrative Code	LBR	Lime rock Bearing Ratio
CIP, C.I.P. or C-I-P	Cast In Place	FC	Friction Course	LF	Linear Foot (Feet)
CJP	Complete Joint Penetration	Fdn.	Foundation	Lgth.	Length
Ckt.	Circuit	F.L. or F	Flow Line	Long.	Longitudinally or Longitudinal
<u> </u>	Center Line	FI.	Florida	LRFD	Load Resistance Factor Design
CI.	Clearance	FDEP	Florida Department Of Environmental Protection	LRS	Low-Relaxation Strand
CMP	Corrugated Metal Pipe	FDOT	Florida Department Of Transportation	LS	Lump Sum
Con.	Connection	FHWA	Federal Highway Administration	LSD	Lump Sum per Day
Conc.	Concrete	FIB	Florida-I Beam	Lt.	Left
Const.	Construct or Construction	F.S.	Florida Statutes		
Cont.	Continuation or Continuous	FS	Far Side		
Corr.	Corrugated	FSB	Florida Slab Beam		
Cov.	Cover	Ft.	Foot or Feet		
CP	Concrete Pipe	FTP	Florida Traffic Plans		

ABBREVIATIONS

FY 2024-25 STANDARD PLANS

bbreviation	Meaning	Abbreviation	Meaning	Abbreviation	<u>Meaning</u>
M m	Meter	 Pen.	P Penetration	: St. or ST.	S
m²	Meter Square	PPB	Pier Protection Barrier	Sta.	Station
Mach.	Machine	PPP	Polypropylene pipe	Std.	Standard
MAS	Motorist Awareness System	Prest.	Prestressed	Stg.	Strong
MASH	Manual for Assessing Safety Hardware (AASHT0)	PRS	Portable Regulatory Sign	Stl.	Steel
Max.	Maximum	psf	Pounds Per Square Foot	SW	Skewed Angle
MES	Mitered End Section	PSI or psi	Pounds Per Square Inch	Swk.	Sidewalk
М.Н.	Manhole or Mounting Height	PT	Point of Tangency or Pressure Treated	SYM	Symmetrical
MHW	Mean High Water	PTFE	Polytetrafluoroethylene		, T
Mid.	Middle	PVC	Polyvinyl Chloride	T or t	Thickness, Tangent Distance or Time
Mil or Mils	One-Thousandth Of An Inch		0	Tan	Tangent
Min.	Minimum or Minute	Q	Flow Volume	T&G	Tongue and Groove
Misc.	Miscellaneous	Qty.	Quantity	TCP	Traffic Control Plan(s)
MLW	Mean Low Water		R	TCZ	Traffic Control Zone
mm	Millimeter	R or Rad.	Radius	Temp.	Temperature or Temporary
Mod.	Modification	Rt.	Right	Theo.	Theoretical
MOT.	Maintenance Of Traffic	R/W	Right Of Way	THW or THWN	Insulation (Flame Retardant, Moisture
MPH or mph	Miles Per Hour	RC	Reverse Crown	TTTW OT TTTWIN	And Heat Resistant Thermoplastic)
MPH OF MIPH MUTCD	Manual On Uniform Traffic Control Devices	RCP	Reinforced Concrete Pipe	TMA	Truck/Trailer Mounted Attenuator
		Rd.	Road or Round	TN	Ton
N					Transition or Transverse
N NA or N/A	Standard Penetration Number	Rdwy.	Roadway Retiguing or Rostangular	Trans. TTC	
NA or N/A	Not Available or Not Applicable	Rect.	Reticuline or Rectangular		Temporary Traffic Control
NC	Normal Crown	Ref.	Reference	TVSS	Transient Voltage Surge Suppression
NCHRP	National Cooperative Highway Research Program	Reinf.	Reinforced or Reinforcement	TX	Transmit
NDCBU	Neighborhood Delivery And Collection Box Unit	Req. or Reqd.	Required	Тур.	Typical
NEMA	National Electrical Manufacturers Association	RGS	Rigid Galvanized Steel		U
NHW	Normal High Water	RPM	Raised Pavement Markers	UL	Underwriters Laboratories
No.	Number	R/R or RR	Railroad	UPS	Uninterruptible Power Supply
Nom.	Nominal	RSDU	Radar Speed Display Unit	USPS	United States Postal Service
NPS	Nominal Pipe Size	RU	Rack Unit	Util.	Utilities
NPT	National Pipe Thread	RX	Receive	UV	Ultraviolet
NS or N.S.	Near Side		5		V
NS	Non-Structural	S or s	Speed, Spacing or Second	Veh.	Vehicle
NTS	Not To Scale	Sch.	Schedule	Vert.	Vertical
O		SHBR	Special Height Bicycle Railing	VPD or Vpd.	Vehicles Per Day
O.C.	On Center	Shldr.	Shoulder		N
0 to 0 or 0.0.	Out to Out	SHW	Seasonal High Water	W	Width or Wide
0.B.G.	Optional Base Group	SIP	Stay In Place	WT	Weight
0D or 0.D.	Outside Diameter	SP	Superpave	WWM	Welded Wire Mesh
0z.	Ounce	Spa., Spcg. or Sp.	Space(ing)(s)	WWR	Welded Wire Reinforcing
P		Spec.	Specification		Υ
Pavt.	Pavement	sq	Square	Yd.	Yard
PBR	Pedestrian/Bicycle Railing	Sq. Ft., SF, sf or S.F.	Square Foot	Yr.	Year
PC	Point Of Curvature	sq. in.	Square Inch		
PCC	Plain Cement Concrete	Sq. Yd., SY or S.Y.	Square Yard		
pcf	Pounds per Cubic Foot	SR	State Road		
PCMS	Portable Changeable Message Sign	FSB	Florida Slab Beam		

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102-220	Temporary Detour Bridge - Steel H Pile Foundations	450-512	Bearing Plates (Type 2) - Prestressed Florida-I & AASHTO Type II Beams
102-230	Temporary Detour Bridge - Steel Pipe Pile Foundations	Structures	Foundations
102-240	Temporary Detour Bridge Thrie - Beam Guardrail	455-001	Square Prestressed Concrete Piles - Typical Details & Notes
<u>Structures</u>		455-002	Square Prestressed Concrete Pile Splices
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400-090	Approach Slabs (Flexible Pavement Approaches)	455-012	12" Square Prestressed Concrete Pile
400-091	Approach Slabs (Rigid Pavement Approaches)	455-014	14" Square Prestressed Concrete Pile
400-289	Concrete Box Culvert Details	455-018	18" Square Prestressed Concrete Pile
400-291	Precast Concrete Box Culverts Supplemental Details	455-024	24" Square Prestressed Concrete Pile
400-292	Standard Precast Concrete Box Culverts	455-030	30" Square Prestressed Concrete Pile
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415-001	Bar Bending Details (Steel)	455-054	54" Precast/Post-Tensioned Concrete Cylinder Pile
415-010	Bar Bending Details (FRP)	455-060	60" Prestressed Concrete Cylinder Pile
Precast Pre	estressed Concrete Construction	455-101	Square CFRP & SS Prestressed Concrete Piles - Typical Details & Notes
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450-045	Florida-I 45 Beam - Standard Details	455-114	14" Square CFRP & SS Prestressed Concrete Pile
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450-072	Florida-I 72 Beam - Standard Details	455-130	30" Square CFRP & SS Prestressed Concrete Pile
450-078	Florida-I 78 Beam - Standard Details	455-154	54" Precast/Post-Tensioned CFRP & SS Concrete Cylinder Pile
450-084	Florida-I 84 Beam - Standard Details	455-160	60" Prestressed CFRP and SS Concrete Cylinder Pile
450-096	Florida-I 96 Beam - Standard Details	Bridge Dec	k Joints
450-120	AASHTO Type II Beam	458-100	Expansion Joint System - Strip Seal
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450-263	Florida-U 63 Beam - Standard Details	460-252	Access Door Assembly for Steel Box Sections
450-272	Florida-U 72 Beam - Standard Details	460-470	Traffic Railing - (Thrie Beam Retrofit) Typical Details & Notes
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460-477	Thrie-Beam Panel Retrofit (Concrete Handrail)			
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521-480	AASHTO Type II Beam			
521-481	Traffic Railing - (Vertical Face Retrofit) Narrow Curb			
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550-010 Bridge Fencing (Vertical)
550-011 Bridge Fencing on Parapet (Curved Top)
550-012 Bridge Fencing (Enclosed)
550-013 Bridge Fencing on Barrier (Curved Top))

Traffic Control Signals and Devices

630-010 Conduit Details - Embedded

Signing, Pavement Markings and Lighting

715-240 Maintenance Lighting for Box Girders

Standard Plan Index	Description
102-110	Sheet 2: Updated the ADHESIVE-BONDING MATERIAL SYSTEM Note to "When using adhesive bonding material systems for anchor bolts, use a Type HSHV adhesive meeting the requirements of Section 937 and listed on the APL. Install anchor bolts in accordance with Specification 416. Field testing requirements of Specification 416 do not apply." Sheet 4: Updated the GEOTEXTILE FABRIC Note: "Provide and install Type D-5 geotextile fabric in accordance with Specifications 514 to contain backfill materials behind the barrier units."
102-200	Sheet 2: Changed reference of "Filter Fabric" to "Geotextile". Sheet 3: Changed reference of "Filter Fabric" to "Geotextile".
102-600	Sheet 1: Added symbols legend. Sheet 9: Added left-turn lane closure detail. Quick Reference Sheet: Updated coefficient in Table 8.
102-660	Increase the maximum work zone speed allowed for Option 2 to 45mph.
120-001	Update Note 4 to reference proper testing.
400-010	Sheet 2: Updated the TYPICAL BACK-FILL DETAIL Callout: " Install a continuous 1.5' x 1.5' drain using clean, broken stone or grave, graded and placed to allow free drainage. Provide and install Type D-3 geotextile in accordance with Specifications 514 around the perimeter to prevent fill from washing out."
400-011	Updated GENERAL NOTES: Note 1 - 3 Updated to active voice. Note 5 - Updated "Joint Seal: Provide and install organic bond breaker or Type D-5 geotextile in accordance with Specifications 400 and 514, respectively." Note 6 - Updated reference from Specification 985 to Specification 514. Changed all references of "Geotextile Fabric" to "Geotextile". Changed all references of "Section" to "Specification".
400-289	Sheet 5: Changed the location of the main reinforcing in Detail J and K to be at the correct point of the radius to maximize the effectiveness of the reinforcing; Changed callout "Filter Fabric" to "Geotextile" in DETAIL "B". Sheet 6: Changed callout "Filter Fabric" to "Geotextile" in DETAIL "L". Sheet 7: Changed callout "Filter Fabric" to "Geotextile" in DETAIL "L" and DETAIL "N". Sheet 8: Changed all references "Filter Fabric" to "Geotextile" and changed Specification reference of 985 to 514 in the ASPHALTIC CONCRETE BASE detail.

Standard Plan Index	Description
400-291	All Sheets: Changed all references of "Filter Fabric" to "Geotextile".
400-292	Sheet 1: Changed all references of "Filter Fabric" to "Geotextile". Sheet 7: Changed all references of "Filter Fabric" to "Geotextile".
415-010	Sheet 1: Added hook styles to the reinforcing list.
425-001	Sheet 1: Updated the Table of Contents to reflect title change on Sheet 3. Sheet 3: Added Note "Wrap with geotextile in accordance with Specification 514." to the PIPE TO STRUCTURE FILTER FABRIC WRAP detail; Changed all references of "Filter Fabric" to "Geotextile". Sheet 4: Added Note 4 "Install Type D-3 geotextile in accordance with Specifications 514." to the SUMP BOTTOM detail.; Changed all references of "Filter Fabric" to "Geotextile".
425-031	Sheet 1: General Note 5 altered to specify "If Alternate G grate is specified in the Plans, field installation of a roller bar is not permitted."
425-052	Moved the note about slot placement from the non-traversable inlet details to General Notes. The note and requirement should apply to all the inlets that have slots.
430-001	Sheet 1: Updated Table of Contents to reflect title change on Sheet 3. Sheet 2: In the ROUND CONCRETE PIPE DETAIL, Deleted Notes 1 and 2; Added New Note 1 "Locate the last full wrap of reinforcement within 3 inches of the spigot shoulder and meet ASTM C76 for round pipe."; Updated all Note references; Updated detail to show 3" "Max" dimension. In the ELLIPTICAL CONCRETE PIPE JOINT DETAIL, Added New Note 1 "Locate the last full wrap of reinforcement within 3 inches of the spigot shoulder and meet ASTM C507 for elliptical pipe."; Updated new Note 2 to "Type D-3 Geotextile Jacket is required on both type of joints."; Updated Note 3 to "Details shown before joint is homed."; Added "3" Max (See Note 1)" callout and dimension to both details. Sheet 3: Updated Note 2 "Install Type D-3 geotextile in accordance with Specification 514. Install securing device to hold the geotextile jacket on to the pipe."; Changed all references of "Filter Fabric" to "Geotextile".

Standard Plan Index	Description
430-012	Sheet 1: Added GENERAL Note 5 "Install Type D-2 geotextile fabric in accordance with Specifications 514." Sheet 2: Changed all references of "Filter Fabric" to "Geotextile".
430-022	General Note 1: Added "or use manufacturer approved coupler" on the end. General Note 2: Added "metal" between corrugated and mitered. Delete "either."
440-001	Sheet 1: Updated Note 5 "Install Type D-3 geotextile fabric in accordance with Specification 514."; Changed all references of "Filter Fabric" to "Geotextile". Sheet 2: Changed all references of "Filter Fabric" to "Geotextile". Sheet 3: Changed all references of "Filter Fabric" to "Geotextile".
443-001	Sheet 1: Updated Note 3 "Install Type D-3 geotextile fabric in accordance with Specification 514."; Changed all references of "Filter Fabric" to "Geotextile". Sheet 2: Changed all references of "Filter Fabric" to Geotextile.
446-001	Sheet 1: Updated Note 4 "Provide and install Type D-3 geotextile fabric in accordance with Specifications 514."; Changed all references of "Filter Fabric" to Geotextile. Sheet 3: Moved Note 3 to General Note 4 on Sheet 1; Renumbered Notes; Updated Note references in callout; Changed all references of "Filter Fabric" to "Geotextile". Sheet 4: Moved Note 3 to General Note 4 on Sheet 1; Renumbered Notes; Updated Note references in callout; Changed all references of "Filter Fabric" to "Geotextile".
450-511	Sheet 1: Changed Dimension L to account for the skew angle; Added Dimension "L" on plan; Added note clarifying negative dimension for AASHTO Beam anchor studs.
455-001	Sheet 1: Corrected typo in Note 6.
455-031	Sheet 1: Changed the location of the dowels; Deleted part of note 2 not allowing for corrugated galvanized metal pipe forms.

Standard Plan Index	Description
455-154	Sheet 1: Corrected Note 3 language for clarity.
455-400	Sheet 1: Changed the "PLASTIC FILTER FABRIC" Note in the SHEET PILE DESIGN CRITERIA AND NOTES to "GEOTEXTILE: Provide and install Type D-2 or D-3 geotextile in accordance with Specifications 514. Extend the geotextile to the bottom of the "X" dimension."; Changed all references of "Filter Fabric" or "Plastic Filter Fabric" to "Geotextile".
455-440	Sheet 1: Changed the "PLASTIC FILTER FABRIC" Note in the SHEET PILE DESIGN CRITERIA AND NOTES to "GEOTEXTILE: Provide and install Type D-2 or D-3 geotextile in accordance with Specifications 514. Extend the geotextile to the bottom of the "X" dimension."; Changed all references of "Filter Fabric" or "Plastic Filter Fabric" to "Geotextile".
458-110	Sheet 1: Changed annotation about form material to indicate it should be removed completely.
509-070	Sheet 1: Change General Note 5, Class VI to 3-4; Delete Type V and Type VI from Traffic Control Devices for Flush Shoulder Roadway Details. Sheet 3: Change 12'-5'' min in PLAN; Change 51" Max to 57" Min in Median Section at Signal Gates Detail.
521-001	Sheet 2: Revised note 5 to accommodate "shoulder rocking".
521-510	Sheet 4: Corrected reinforcing steel estimated quantities.
521-511	Sheet 1: Updated references to notes. Sheet 3: Corrected reinforcing steel estimated quantities.
521-513	Sheet 2: Corrected reinforcing steel estimated quantities.
521-650	Sheet 1: Table 1 - Changed the column headings to "Top of Pedestal Height (Ft.)" and "Luminaire Mounting Height"; Note 10 - Added Index 521-611 reference. Sheet 3: Detail "A" - Added minimum depth for base plate; Added new Anchor Plate Detail.

Standard Plan Index	Description
524-001	Sheet 1: Updated GENERAL NOTE 5 "Select appropriate geotextile based on the application type referenced in Specification 985 and install in accordance with Specification 514 under all ditch pavement except for miscellaneous asphalt." Sheet 2: Changed "Filter Fabric" to "Geotextile" in Table 1. Sheet 3: Changed all references of "Filter Fabric" to "Geotextile".
536-001	ALL Sheets: Renumbered all notes and sheet borders to account for new Sheet 21. Sheet 1: Updated Table of Contents to add new Sheet 21; Modified General Note 4 to remove the need for washers at panel splices and steel post flanges. Sheet 5: Removed Modified Thrie-Beam reference in Note 6 Sheet 18: Revised to increment Sections F-F to H-H and G-G to I-I Sheet 19: Revised to increment Section H-H to J-J Sheet 20: Added new reference for trailing end curb options in Note 1 Sheet 21: New Sheet Added new Index Sheet for "Trailing End Transition Connection to Rigid Barrier - Curb Connections" (Curb Options).
546-001	Removed the option for stacking multiple applications of preformed thermoplastic to achieve a height of 1/2" for Raised Rumble Strips. Included a 6" offset from the end of the rumble strips to lane lines.
550-011	All Sheets: Change Index title.
550-012	Sheet 1: Replaced the 7'-5" dimension because the calculated dimension is off by 3/8" and the dimension varies depending on clear width; Removed the 5'-5" dimension because it varies; Revised the Clear Width note from 5ft min to add 12 ft max.
550-013	All Sheets: Change Index title.
630-010	Sheet 1: Revised conduit call out to remove material specification.
639-001	Sheet 1: Added pole dimensions for the new Type P-IID concrete pole option in Detail B; Removed height dimensions for the meter and service disconnect in Details A and B; Added reference to Note 6 in the meter and service disconnect callouts in Details A and B; Added Note 5 to reference Index 641-010 for prestressed concrete pole details and explain optional pole usage; Added Note 6 to provide height information for the meter and service disconnect.

Standard Plan Index	Description
641-020	Sheet 2: Added note allowing additional strand pattern and updated associated tables. Sheet 3: Added a new strand pattern.
646-001	Added Note 6, "In lieu of footing design shown, a Spread Footing may be used in accordance with index 700–120."
649-010	Sheet 1: Changed phrase from "full-penetration groove" to "complete joint penetration; Changed note 1 and revised Section A-A to add clarity to anchor bolt nuts. Sheet 3: Changed weld detail to say "CJP."
649-020	Sheet 1: Changed phrase from "full-penetration groove" to "complete joint penetration. Sheet 3: Changed note 2 and revised Section A-A to add clarity to anchor bolt nuts. Sheet 4: Changed weld detail to say "CJP."
649-031	Sheet 1: Changed phrase from "full-penetration groove" to "complete joint penetration." Sheet 2: Changed note 3 and revised Section A-A to add clarity to anchor bolt nuts. Sheet 3: Added a note about the diameter of the female splice. Sheet 4: Changed weld detail to say "CJP." Sheet 6: Changed weld detail to say "CJP."
654-001	Update Note 4; Added Note 12, "In lieu of footing design shown, a Spread Footing may be used in accordance with index 700-120."
676-010	All Sheets: Renumbered Sheet 1: Deleted Existing Controller Cabinet detail and Notes Sheet 3: Updated Metal Pole to have an ITS Cabinet Sheet 4: New sheet for Ground Mounted Controller Cabinet Riser.

Standard Plan Index	Description
695-001	Sheet 1: Added modem antenna to cabinet; Removed "furnished separately" from Backplane callout; Moved J1 Mounting Bracket Details to Sheet 18; Added Table of Content; Updated Note 1D from "J1" to "P1"; Updated Note 4 to read "Provide and install Speed/Classification Unit, Modem, and Antenna"; Updated Note 5 to be consistent with other sheets; Updated Note 6 to read "Provide and install a 12-fiber single mode cable, a 12-port patch panel, and a managed field Ethernet switch." Titled sheet Continuous Count Station Traffic Monitoring Site - TTMS/CCS." Sheet 2: Added modem antenna to cabinet; Removed "furnished separately" from Backplane callout; Updated Note 1D from "J1" to "P1"; Updated Note 4 to read "Provide and install Speed/Classification Unit, Modem, and Antenna."; Update Note 5 to be consistent with other sheets; Updated Note 6 to read "Provide and install a 12-fiber single mode cable, a 12-port patch panel, and a managed field Ethernet switch." Titled Sheet "Continuous Count Station Traffic Monitoring Site - TTMS/CCS"; Changed Veh. Sensor Term. Strip to Piezo Sensor Terminal Strip. Sheet 3: Titled sheet for Continuous Count Station Traffic Monitoring Site - TTMS/CCS"; Changed Veh. Sensor Term. Strip to Piezo Sensor Terminal Strip. Sheet 4: Titled sheet for Continuous Count Station Traffic Monitoring Site - TTMS/CCS; Titled details "Pinout Chart, Receptacles, and Plug Details"; Corrected Equipment Cable Plug table to P1. Sheet 5: Added "Traffic Flow" in each lane; Updated Note #3 to read "Twist loop leads at the rate of 8 to 16 twists per foot. Extend the twisted pair loop wire directly to the cabinet. No splicing of the loop leads will be permitted." Deleted Note #6; Moved DETAILS "A THRU 'C' to Sheet 18; Titled sheet "Continuous Count Station Traffic Monitoring Site - TTMS/CCS - Lane Layout for TTMS/CCS Inductive Loop and Axle Sensors"; Updated callouts to specify "non weight" axle sensors; Renumbered notes. Sheet 6: New Sheet 11 - Weigh-In-Motion Monitoring Site - PTMS - Cabinet Layout Details (Four

Standard Plan Index	Description
695-001 Cont.	Sheet 8: New Sheet 19 - Non-Intrusive Vehicle Sensor; Added Note 4 - "Cabinet, ground rod pull box, and maintenance service slab installed per Index 676-001, except cabinet center will be 4 feet above grade."; Added Maintenance Service Slab and Pull Box for ground rod to the ELEVATION detail. NEW Sheet 8: Short Term Traffic Monitoring Site - PTMS - Lane Layout for TMS Inductive Loop and Axle Sensors. Sheet 9: New Sheet 20 - Added Title "Solar Power Pole with Pole Mounted Cabinet and Pedestal Mounted Cabinet Details"; Updated Note 1 to read "Cabinet, ground rod pull box, and maintenance service slab installed per Index 676-001, expect cabinet center will be 4 feet above grade."; Added Maintenance Service Slab and Pull Box for ground rod to the two ELEVATION details; Deleted ground rod clamps and added exothermic weld call out; Updated the DETAILS lettering sequence. NEW Sheet 9: Weigh-In-Motion Monitoring Site - Cabinet Layout Details (Four Lanes or Less). Sheet 10: New Sheet - Weigh-In-Motion Monitoring Site - Cabinet Backplane Details. Sheet 11: New Sheet - Previous Sheet 6. Sheet 12: New Sheet - Non-Motorized Monitoring Site - Cabinet Sideplane and Cabinet Backplane Details. Sheet 13: New Sheet - Non-Motorized Monitoring Site - Large Shared Use Path Configurations. Sheet 16: New Sheet - Non-Motorized Monitoring Site - Extra Large Shared Use Path Configurations. Sheet 17: New Sheet - Non-Motorized Monitoring Site - Paved Sidewalk Configuration. Sheet 18: New Sheet - Non-Motorized Monitoring Site - Paved Sidewalk Configuration. Sheet 18: New Sheet - Previous Sheet 8. Sheet 20: New Sheet - Previous Sheet 9.
700-010	Sheet 5: Change minimum Driven Post Hole Diameter to 12" and added Note 2 about an alternate post hole shape. Sheet 9: Changed second sign from top on 3rd column (right column from 2 digit US Shield to 3 digit US Shield. Changed sign image.

Standard Plan Index	Description
700-011	Changed the first column to have sign size of Width x Height rather than Height x Length. The four dimensions under the heading will need to be reversed to make this change complete.
700-020	Sheet 1: Revised bolts in the Hanger Beams to show correctly; Changed all reference of length to width and depth to height. Sheet 2: Added Foundation Notes to allow for precast concrete foundation with an octagon shape as a substitute for the circular shaped foundation shown. Sheet 3: Revised bolts in the Hanger Beams to show correctly; Changed all reference of length to width and depth to height.
700-030	Sheet 1: Corrected CADD error not showing the bolts in the Hanger Beams correctly; Added language to Detail B bolt call out to clarify number of bolts required; Changed all reference of length to width and depth to height.
700-090	Sheet 4: Changed weld detail to say "CJP."
700-101	Add reflective strip to post on WWD sign Case X.
700-102	For signs $G20-1$ and $G20-2$ changed the dimension from height x width to width x height $(4'x2')$.
700-110	Added missing callout; Added additional callouts for clarity: Changed all reference lengths to width to keep consistent with MUTCD.
700-120	Sheet 3: New Sheet - Spread Footing Foundation Sheet 12: New Sheet - Roadside Sign Assembly - 9 Sheet 13: New Sheet - Roadside Sign Assembly - 10 Sheet 14: New Sheet - Roadside Sign Assembly - 11
711-003	Sheet 1: Deleted 18" white hash marks.
715-001	Sheet 1: Deleted the "Access Panel" drawing and added a "Pole Base" callout on the 'Metal Pole Wiring Detail'. Added the "Access Door" drawing and a "Pole Base" callout on the 'Metal Pole Detail'.

Standard Plan Index	Description
715-002	Sheet 4: Added Shaft Foundation Note 3 to allow for concrete foundation with an octagon shape as a substitute for the circular shaped shaft that's shown. Sheet 5: Added new structural grout pad details to the plan and section view. Added new note 9 to explain structural grout pad construction and reference specification. Sheet 6: Changed weld detail to say "CJP."
715-003	Sheet 3: Added Foundation Note 3 to allow for precast foundation with an octagon shape as a substitute for the circular shaped shaft foundation shown.
715-010	Sheet 1: Changed phrase from "full-penetration groove" to "complete joint B70penetration. Sheet 2: Added Table numbers. Sheet 3: Corrected the welding arrows on Section A-A and C-C to be consistent with AWS; Added note to Section CC that silicone caulk should be done after galvanizing; Changed weld detail to say "CJP."

STRUCTURAL STEEL:

Steel Plates and Rolled Sections shall be ASTM A709 Grade 36. Pipe piles shall be ASTM A252 Grade 2, Fy = 35 ksi.

BOLTS, LAG SCREWS AND THREADED BOLT STOCK:

Furnish high strength bolts in accordance with ASTM F3125 Grade A325 Type 1. Furnish Threaded Stock in accordance with ASTM A36. Furnish Lag Screws in accordance with ASTM A307. Furnish steel washers and nuts compatible with Bolts, Threaded Stock and Lag Screws.

TIMBER AND LAGGING:

Timber and Lagging shall be No. 1 Southern Yellow Pine.

Work this Index with Index 102-210, 102-220, 102-230 and 102-240.

BACKWALL BENT PILES:

Timber Piles:

10' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6). Ultimate Capacity greater than 18 tons.

Splices are not allowed on any timber piles.

H-Piles:

12' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6).

Ultimate Capacity greater than 18 tons.

Shims admissible between backwall pile and cap. Test piles are not required for backwall piles.

EXPANSION BEARINGS:

Inspect the PTFE (Teflon) layer and stainless steel plate prior to installation. Do not use bearings that have a severely damaged or unbonded PTFE layer. Clean PTFE of all grit and grime prior to installation. Clean Stainless steel plate of all grit and grime prior to installation and finish to a smooth buffed surface.

DISTRIBUTING BEAMS:

Longitudinal stops restraining the distributing beams may be lengthened or shortened to center the distributing beam bearing on the cap beam.

The longitudinal stops are to bear on the distributing beam end frame.

EXPANSION JOINT SETTINGS:

Install the expansion joint considering the total continuous bridge length, location of fixed bearings and ambient temperature at the time of installation, assume a 1" expansion joint opening at 70 degrees F.

STORAGE FACILITY:

Contact
FDOT Statewide Aluminum Shop
2590 Camp Rd.
Oviedo, Fl.
407-278-2727

For shipping weights and dimensions of Temporary Bridge elements.

SHIPPING WEIGHTS AND DIMENSIONS:

Decking Sizes:

Туре	Length	Width	Weight (lbs.)
Curb	5'	6'-9"	800
Curb	10'	6'-9"	1420
Curb	15'	6'-9"	2200
Curb	20'	6'-9"	2800
NonCurb	5'	5'-3"	650
NonCurb	10'	5'-3"	1000
NonCurb	15'	5'-3"	1600
NonCurb	20'	5'-3"	2100

Shipping weights and dimensions of other bridge components can be referenced in "Acrow Panel Bridging, Series 300, Technical Handbook".

TRAFFIC RAILING NOTES:

See Index 536-001 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements of AASHTO M180, Type II (Zinc coated).

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Do not drill Temporary Bridge components to attach Guardrail. Guardrail Bolts shall be placed between Truss members as shown in Index 102-240.

COATINGS: All Nuts, Bolts, Anchors, Washers and Backer Plates shall be hot-dip galvanized in accordance with the Specifications.

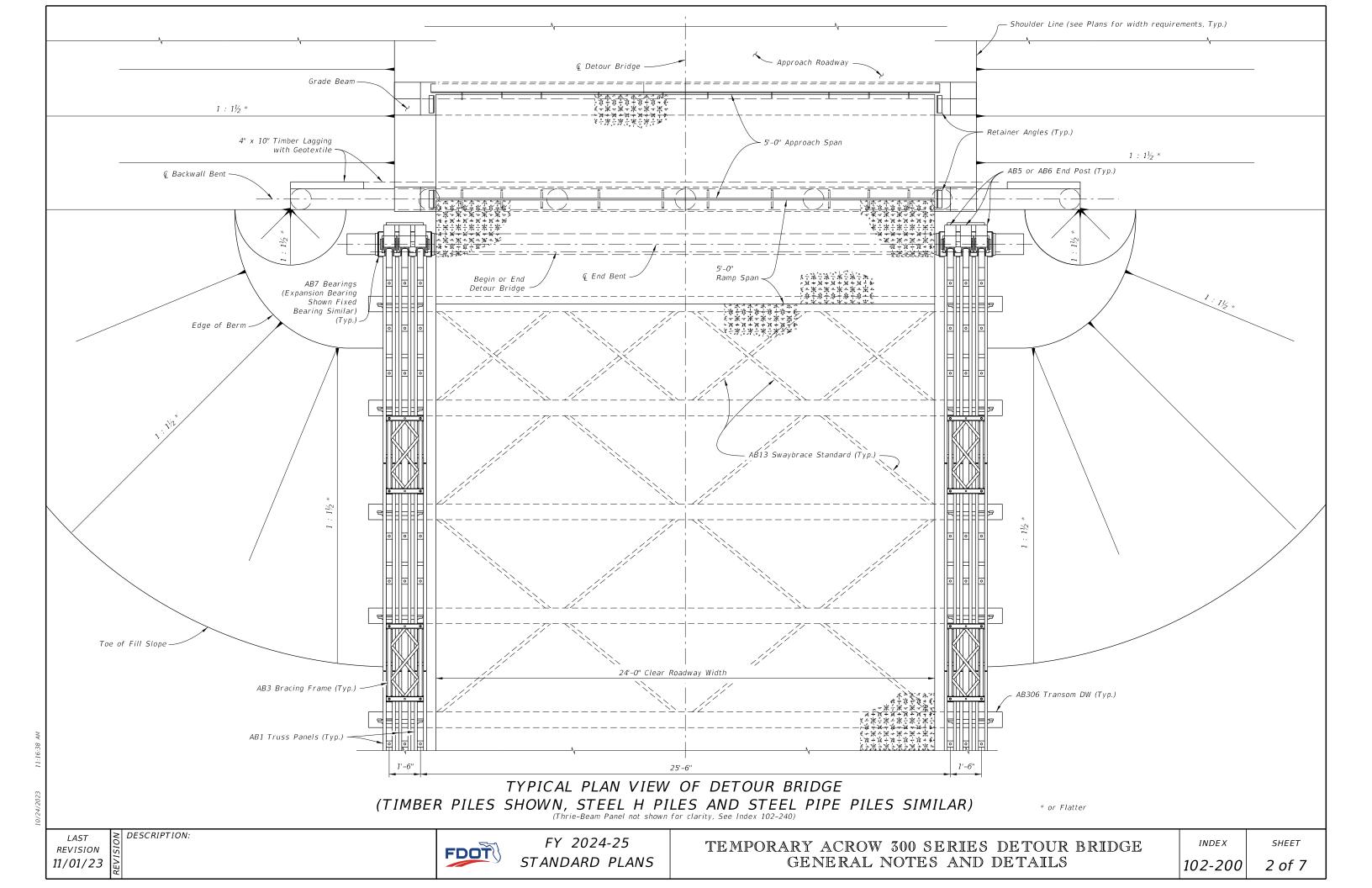
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered $(\pm 1/4")$.

PAYMENT:

Temporary Detour Bridge is to be paid for under Contract Unit Price for Special Detour. If a temporary bridge system other than that shown herein is used, the Contractor is responsible for renting or purchasing their own system. Payment for Temporary Guardrail work and Transition Block will be made under Pay Item Temporary Guardrail, LF.

Furnish and install Bridge Thrie-Beam Panels and all associated hardware as shown. Payment will be made with the Temporary Detour Bridge under the Pay Item Special Detour, LS. Turn over Bridge Thrie-Beam Panels and all associated hardware to the Department with the Detour Bridge components per Specifications Section 102-6.

102-200



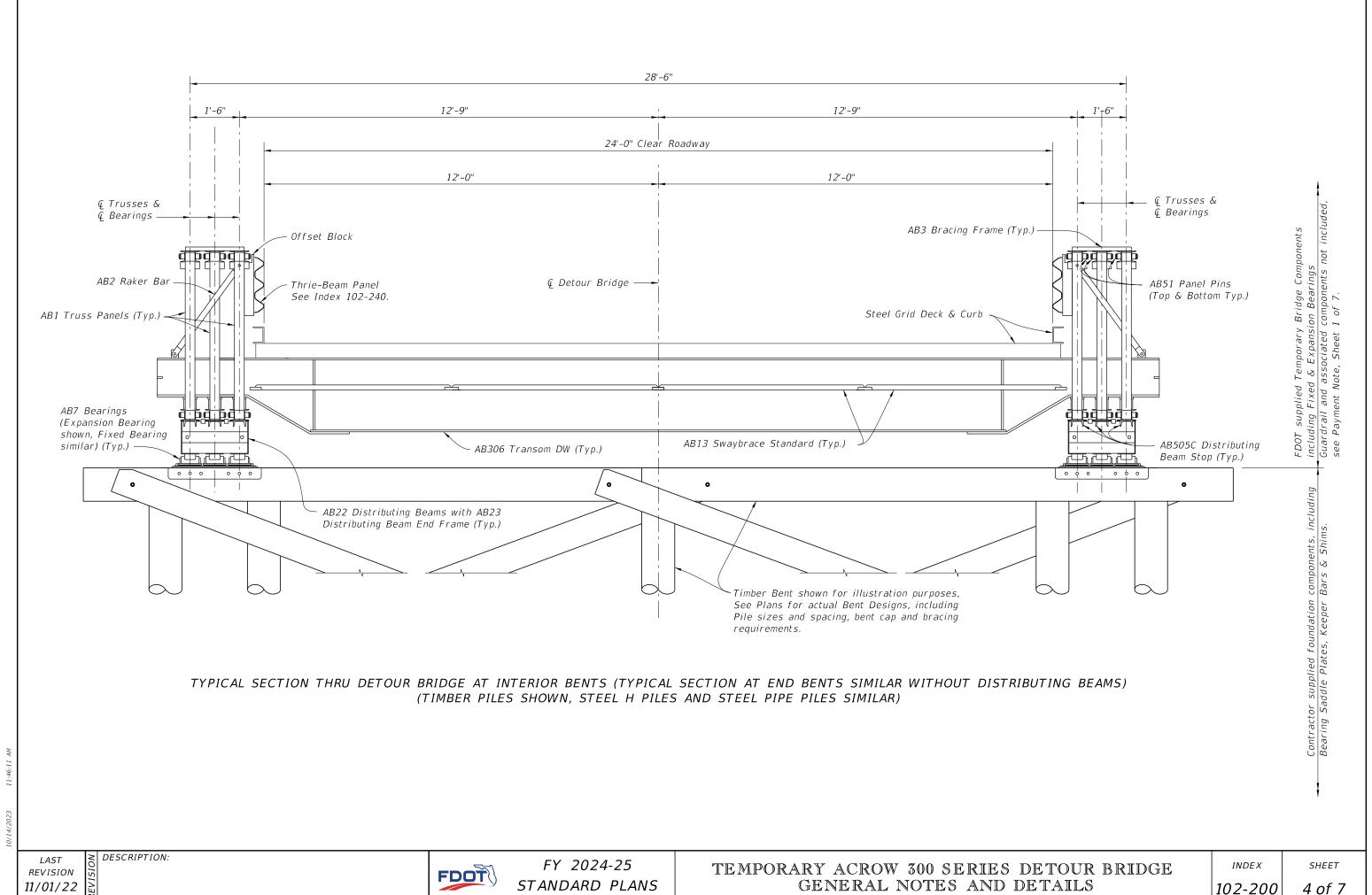
ELEVATION VIEW (TIMBER PILES SHOWN, STEEL H PILES AND STEEL PIPE PILES SIMILAR)

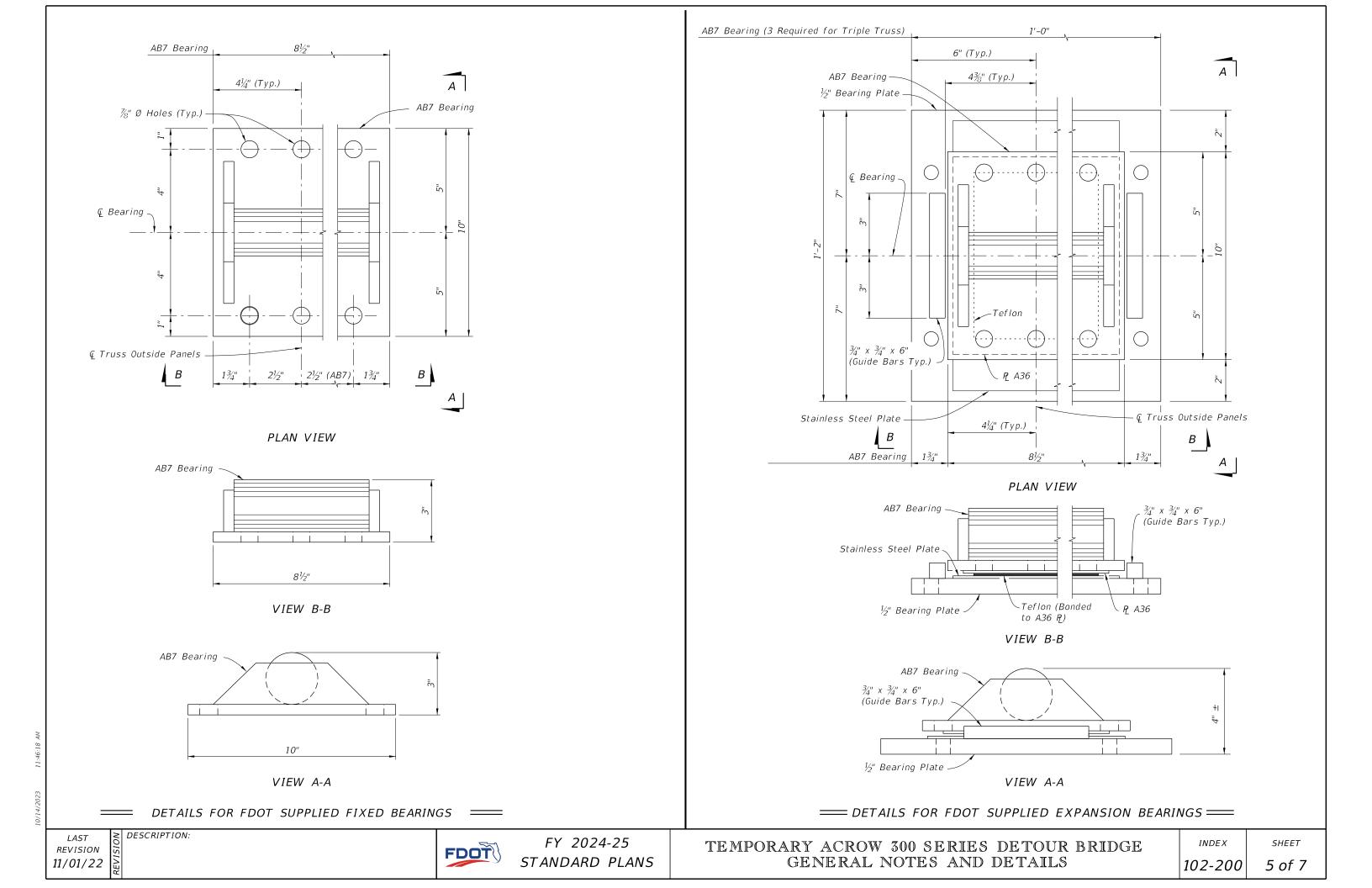
(Thrie-Beam Panel not shown for clarity, See Index 102-240)

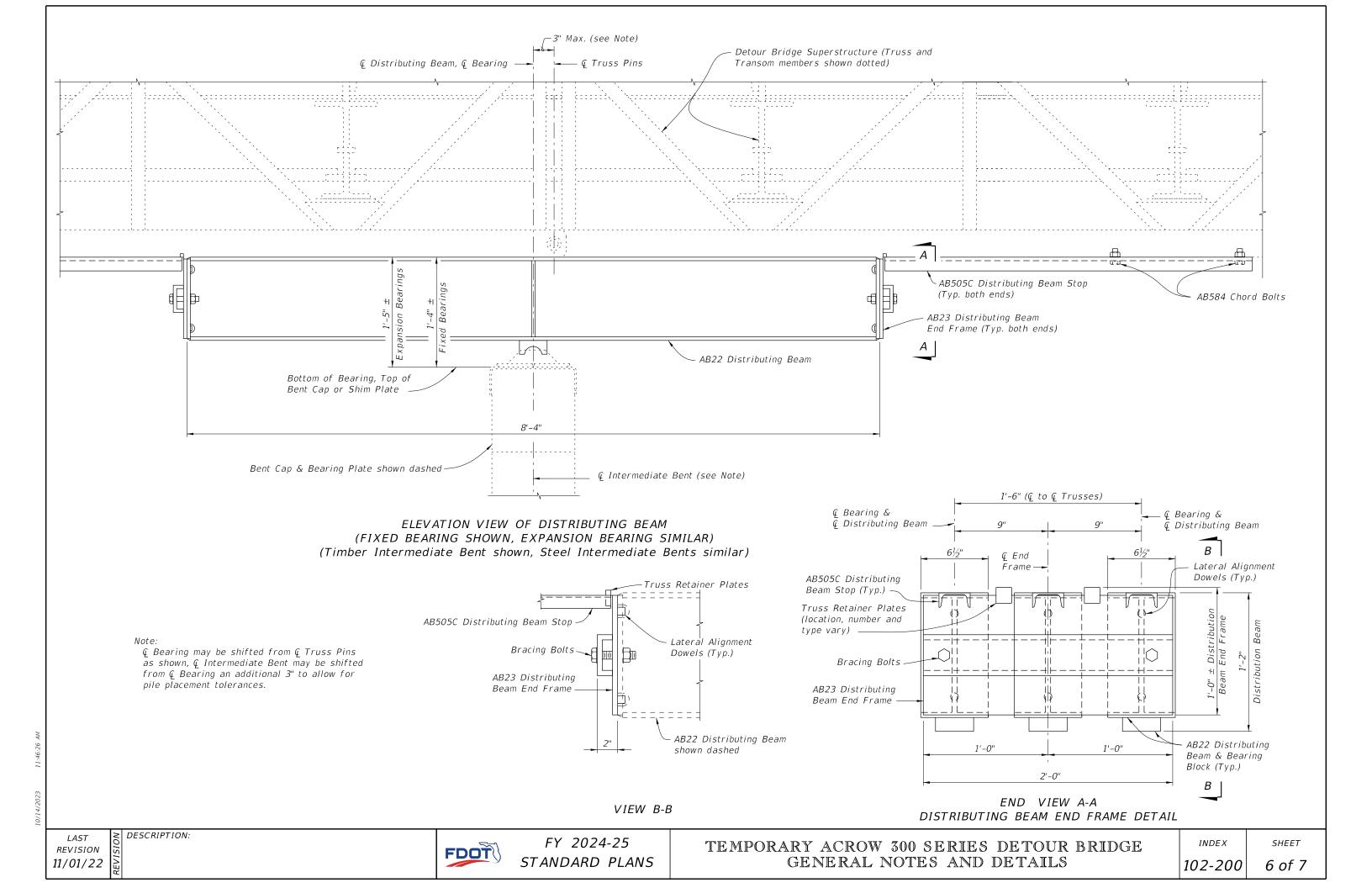
REVISION 11/01/23

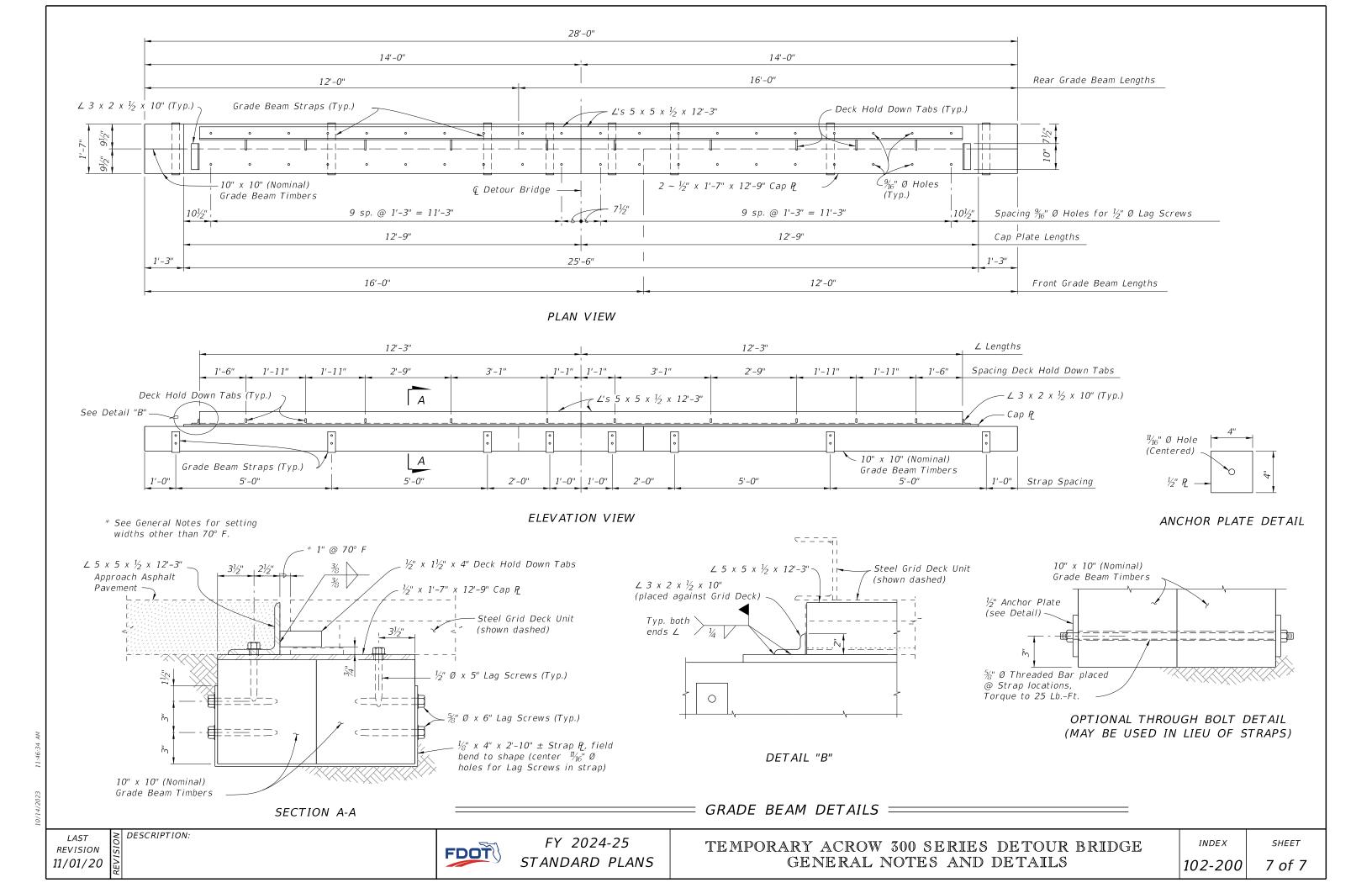
DESCRIPTION:

FDOT









Work this Index with Index 102-210, 102-220 and 102-230.

STRUCTURAL STEEL:

Steel Plates shall be ASTM A709 Grade 36.

EXPANSION BEARINGS:

Inspect the PTFE (Teflon) layer and stainless steel plate prior to installation. Do not use bearings that have a severely damaged or unbonded PTFE layer. Clean PTFE of all grit and grime prior to installation.

Clean Stainless steel plate of all grit and grime prior to installation and finish to a smooth buffed surface.

DISTRIBUTION BEAMS:

Distribution beam stops restraining the distribution beams may be lengthened or shortened to center the distributing beam bearing on the cap beam.

The longitudinal stops are to bear on the distributing beam end frame.

EXPANSION JOINT SETTINGS:

Install the expansion joint considering the total continuous bridge length, location of fixed bearings and ambient temperature at the time of installation, assume a 2" expansion joint opening at 70 degrees F, (Expansion joint depends on span/bridge length and configuration).

STORAGE FACILITY:

Contact

FDOT Statewide Aluminum Shop 2590 Camp Rd. Oviedo, Fl. 407-278-2727

For shipping weights and dimensions of Temporary Bridge elements.
Contractor to coordinate with Storage Facility and Acrow to obtain required parts list.
Shipping weights and dimensions of other bridge components can be
referenced in "Acrow Panel Bridging, Series 700XS, Technical Handbook".

APPROACH TRAFFIC RAILING NOTES:

See Index 536-001 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements of AASHTO M180, Type II (Zinc coated).

BOLTS, NUTS AND WASHERS:

DESCRIPTION:

Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Do not drill Temporary Bridge components to attach Guardrail. Guardrail Bolts shall be placed between Truss members as shown in Index 102-240.

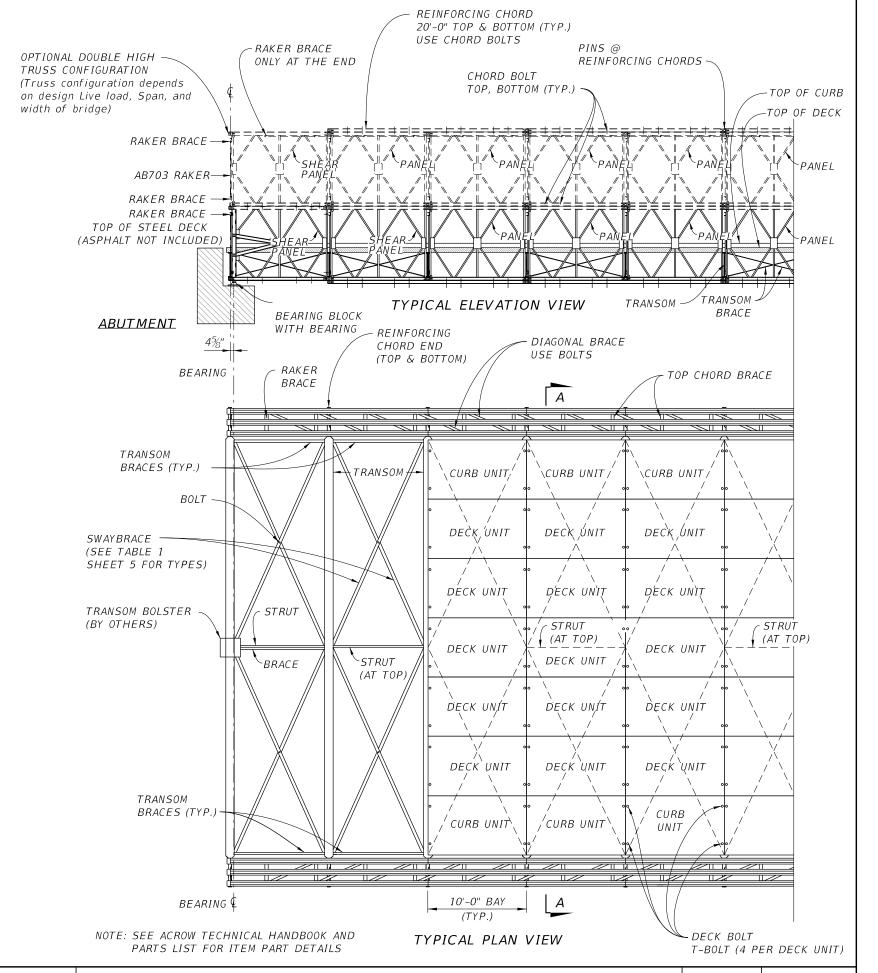
COATINGS: All Nuts, Bolts, Anchors, Washers and Backer Plates shall be hot-dip galvanized in accordance with the Specifications.

WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered $(\pm 1/4")$.

PAYMENT:

Temporary Detour Bridge is to be paid for under Contract Unit Price for Special Detour. If a temporary bridge system other than that shown herein is used, the Contractor is responsible for renting or purchasing their own system. Payment for Temporary Guardrail work and Transition Block will be made under Pay Item Temporary Guardrail, LF.

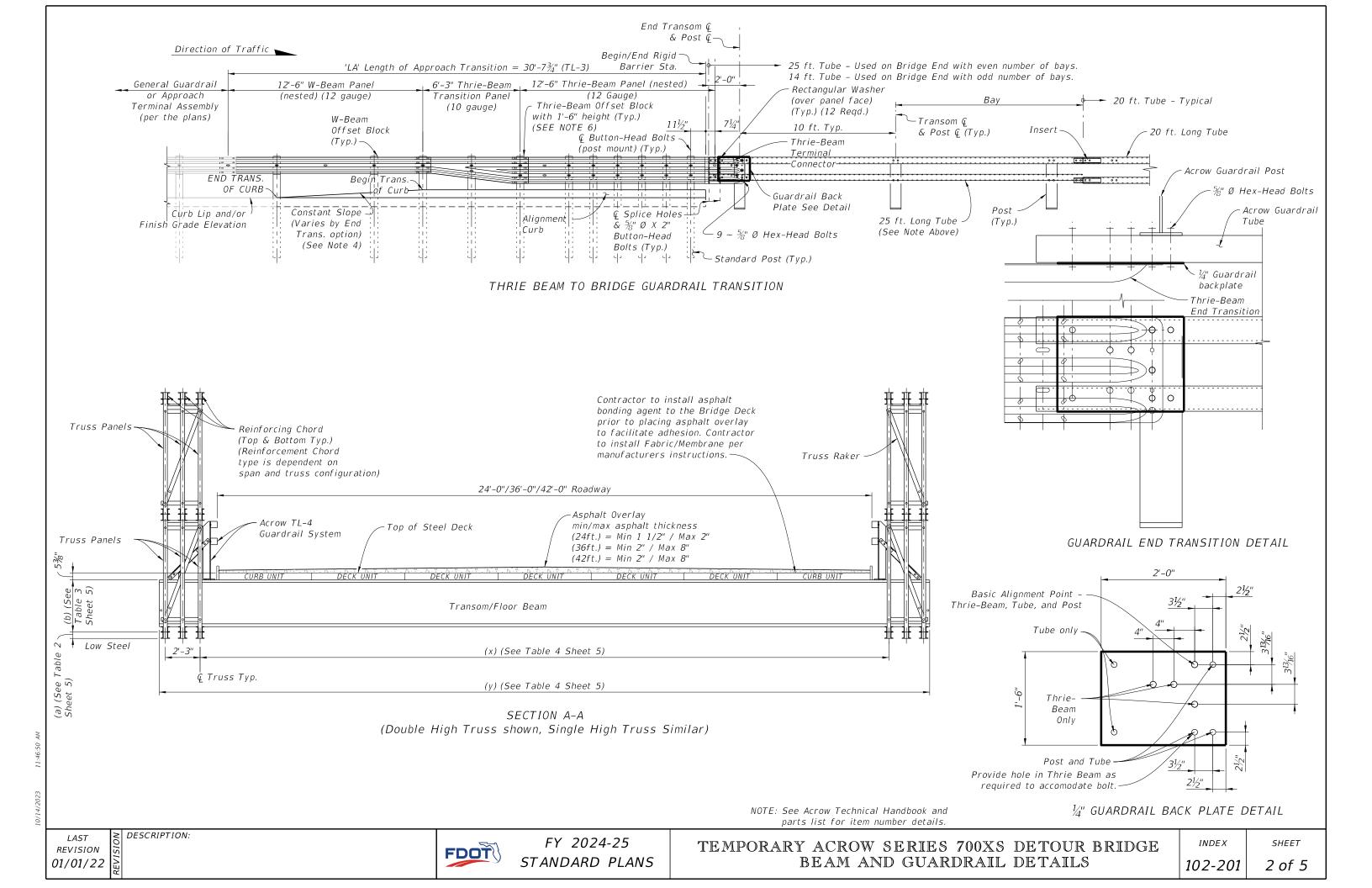
Furnish and install Bridge Thrie-Beam Panels and all associated hardware as shown. Payment will be made with the Temporary Detour Bridge under the Pay Item Special Detour, LS. Turn over Bridge Thrie-Beam Panels and all associated hardware to the Department with the Detour Bridge components per Specifications Section 102-6.

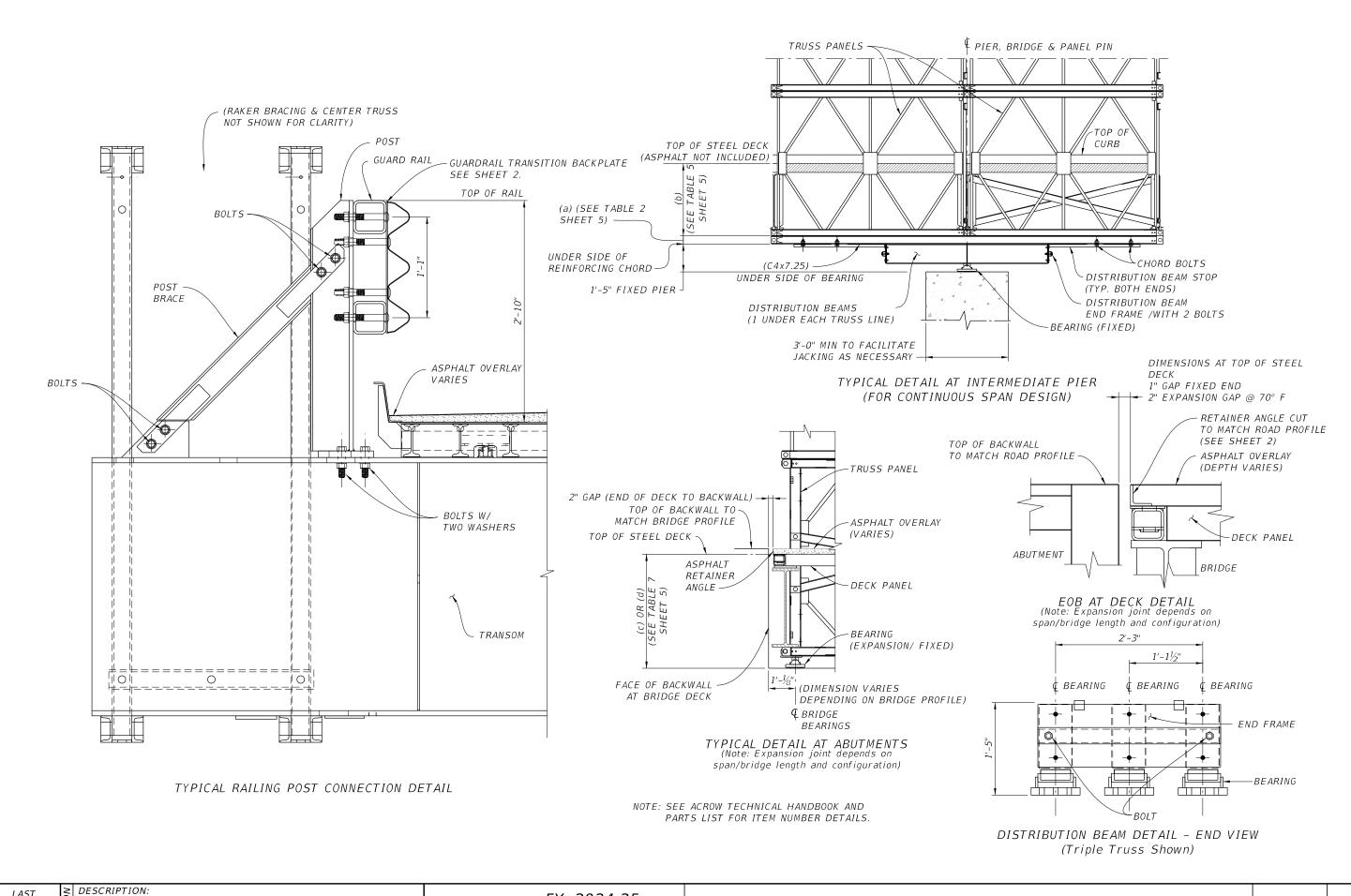


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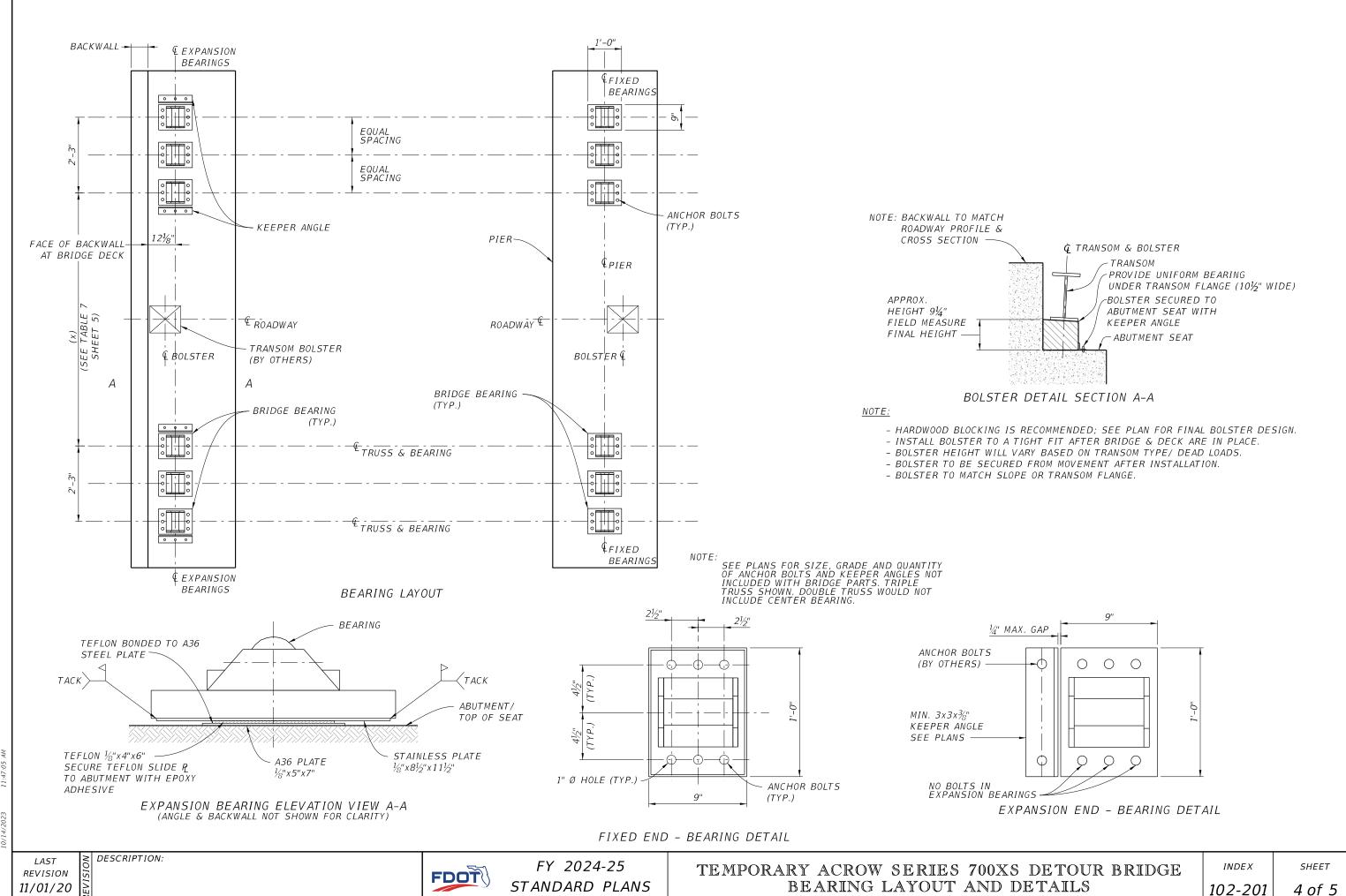
FY 2024-25 STANDARD PLANS





LAST REVISION 11/01/20

FDOT



STANDARD PLANS

102-201

TABLE 1 Swaybrace / Transom Brace				
Bridge Swaybrace Swaybrace Transom Roadway Transom Part # Part # Brace width (ft) (Single) (Double) Part #				
24	SC0017	AB590	AB515	AB519
36	AB957	AB891	AB891	AB519
42	AB978	AB979	AB979	AB519

TABLE 2				
(a) Reinfo	orcing Chord	Thickness		
Regular Heavy SuperHeav Reinforcing Reinforcing Reinforcing Chord Chord Chord Thickness Thickness Thickness				
4"	5"	6"		

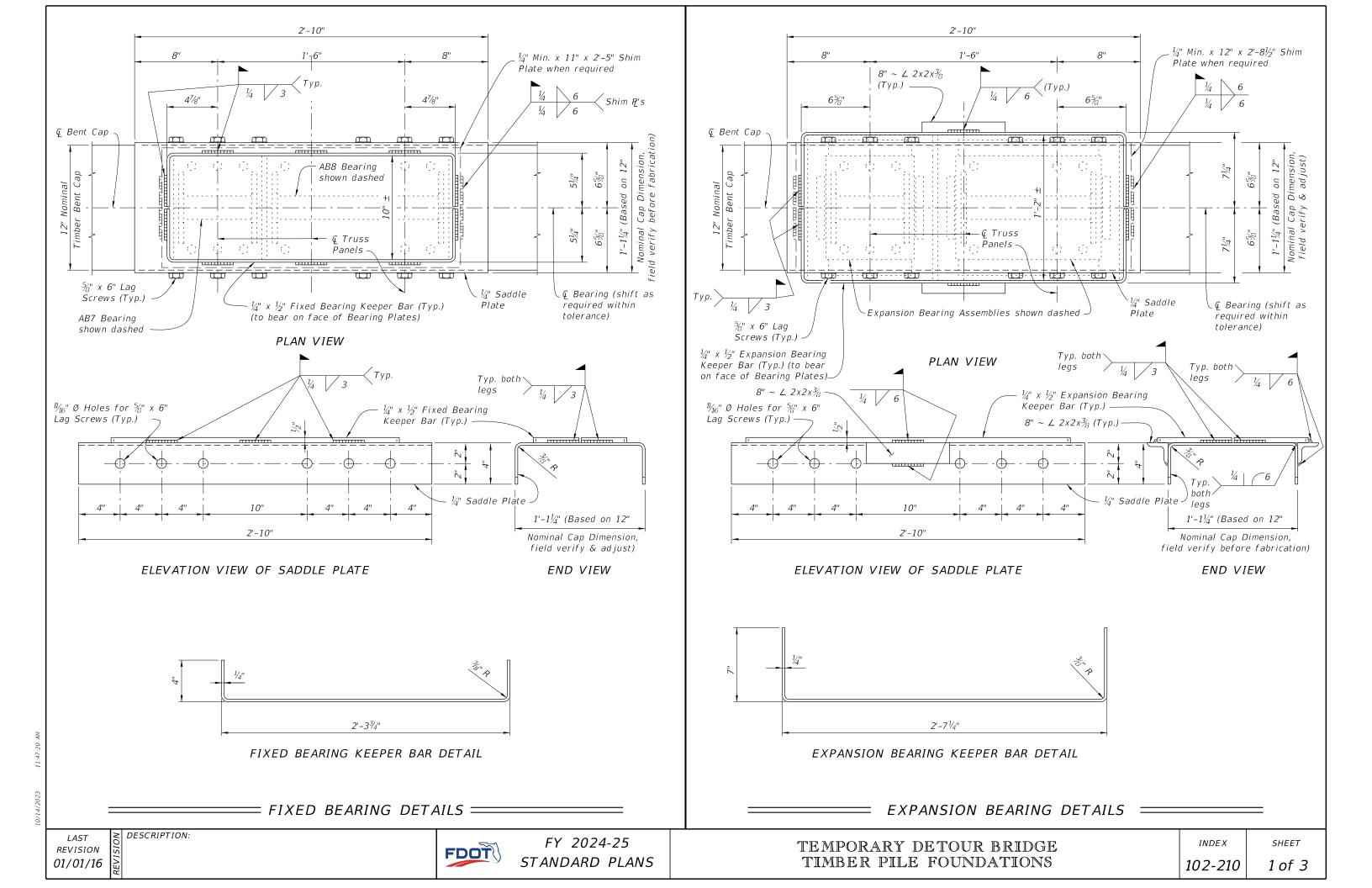
TABLE 3			
Bridge Roadway width (ft)	Transom Part #	(b) Height Bottom of Truss Chord to top of Transom	
24	SC0017	285⁄ ₁₆ "	
36	AB957	40¾6"	
42	AB978	43"	

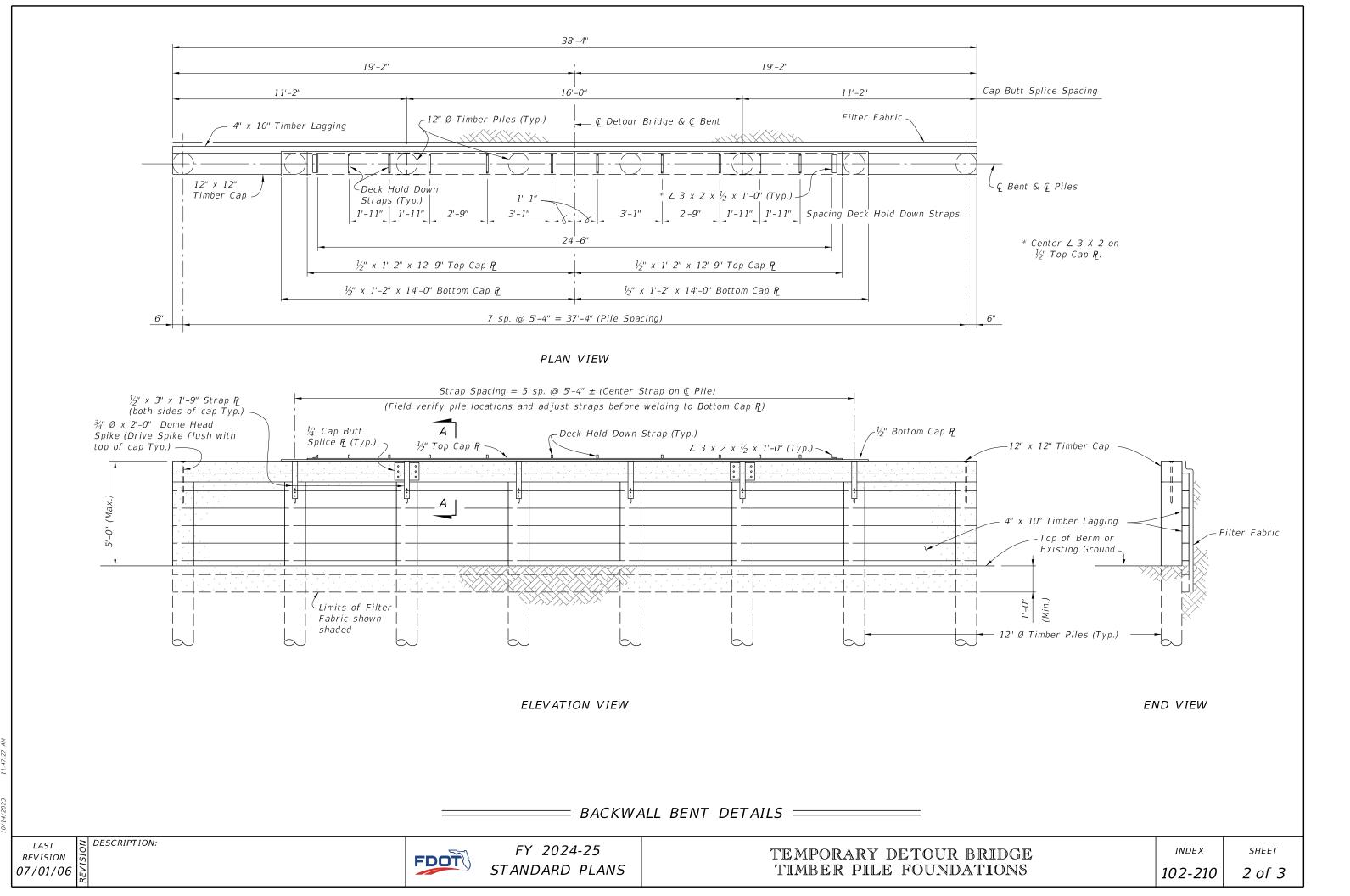
TABLE 4				
Bridge Roadway width (ft)	Transom Part #	(x) Q to inner truss to Q inner truss	(y) Transom Beam Length	
24	SC0017	26'-1"	31'-4"	
36	AB957	38'-4 ¹³ / ₁₆ "	43'-7 ¹³ / ₁₆ "	
42	AB978	44'-43/8"	49'-7 % "	

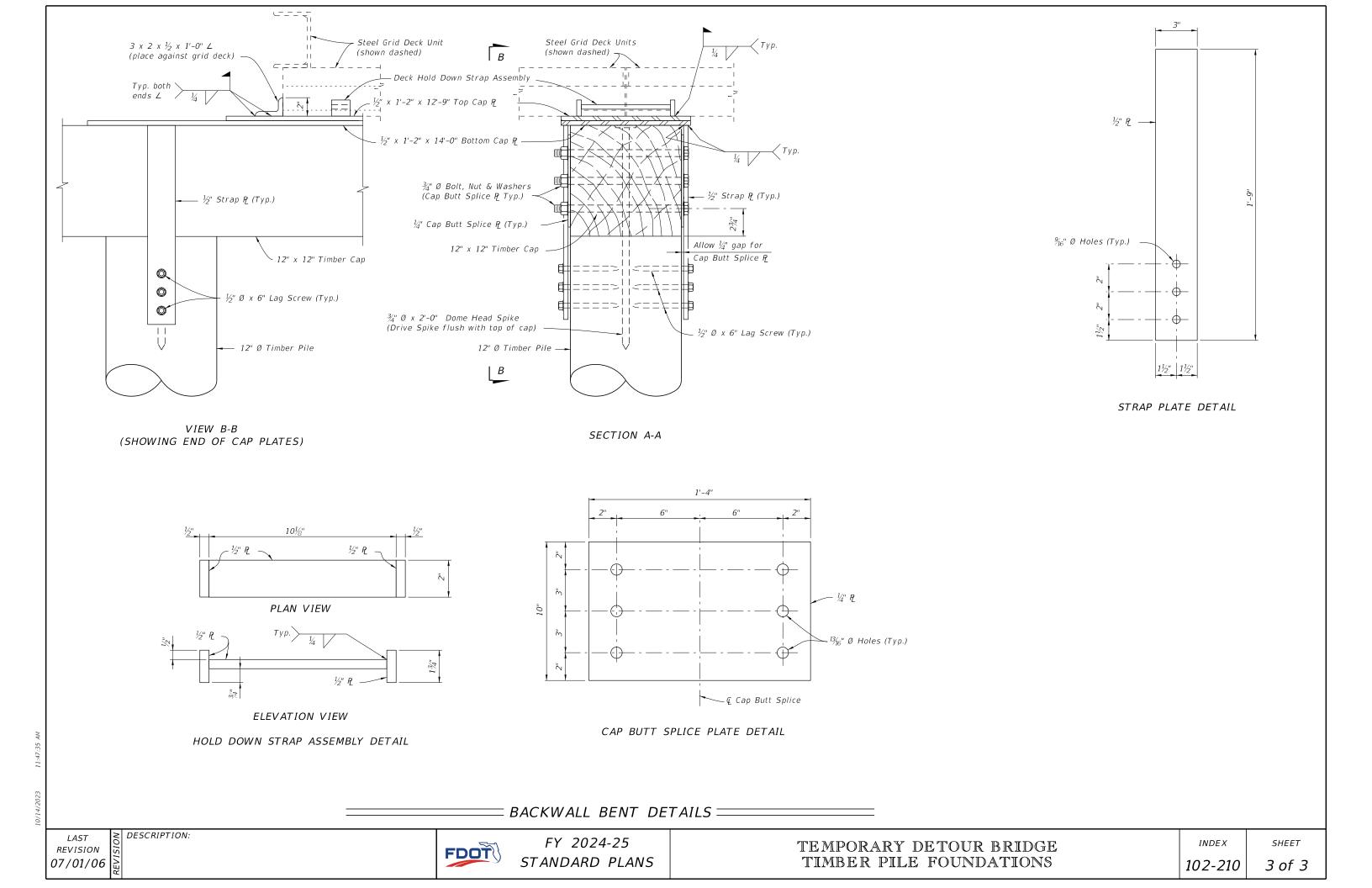
TABLE 5			
Bridge Roadway width (ft)	Transom Part #	(b) Height Bottom of Truss Chord to top of Deck	
24	SC0017	33 ¹¹ / ₁₆ "	
36	AB957	45% <u></u> "	
42	AB978	48 % "	

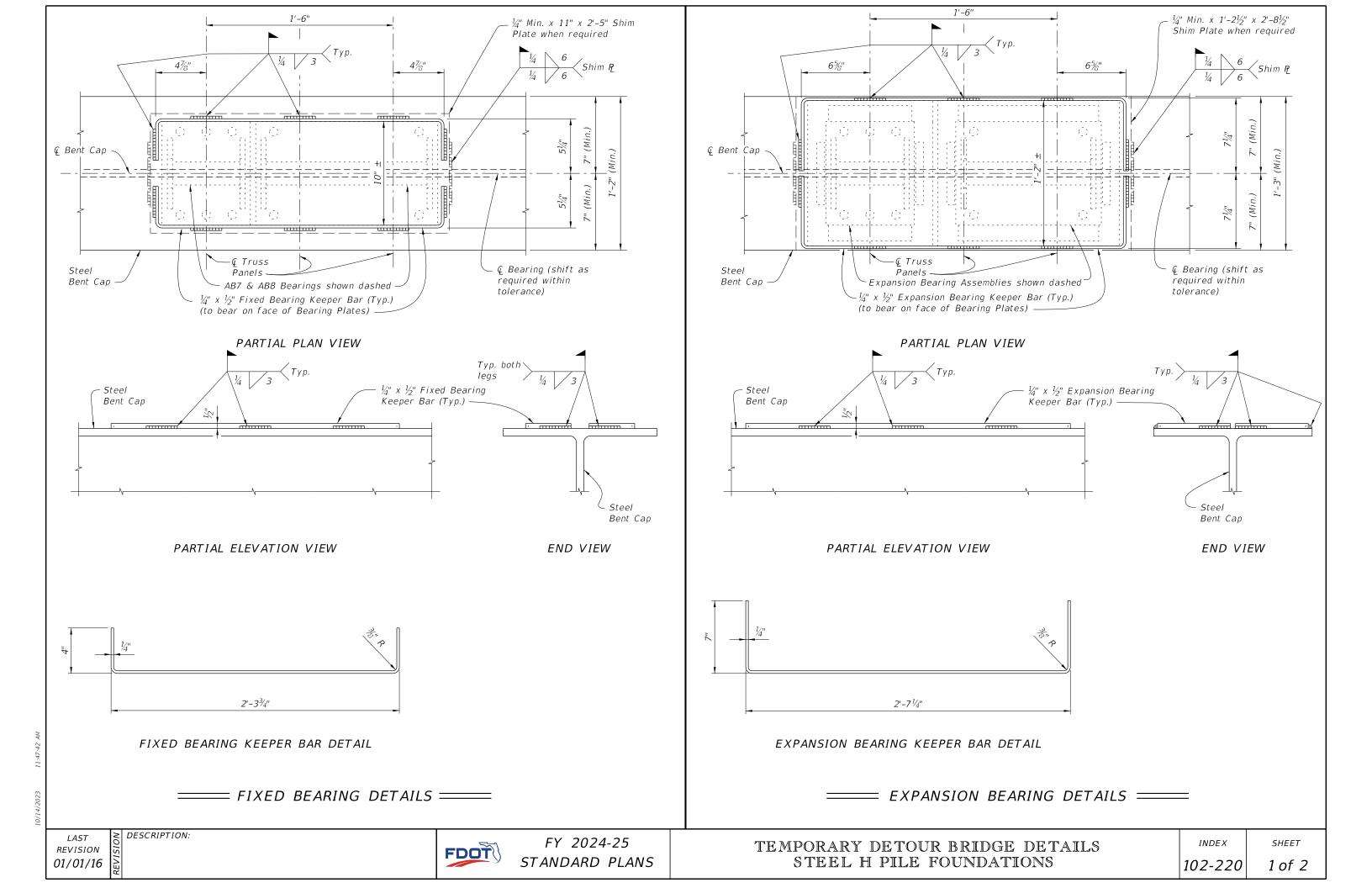
TABLE 6				
Bridge Roadway width (ft)	Transom Part #	(c) Height Bottom of fixed bearing to top of Deck	(d) Height Bottom of expansion bearing to top of Deck	
24	SC0017	39½ _{6"}	39½"	
36	AB957	50 ¹⁵ / ₁₆ "	51 ⁵ / ₁₆ "	
42	AB978	53¾"	54½"	

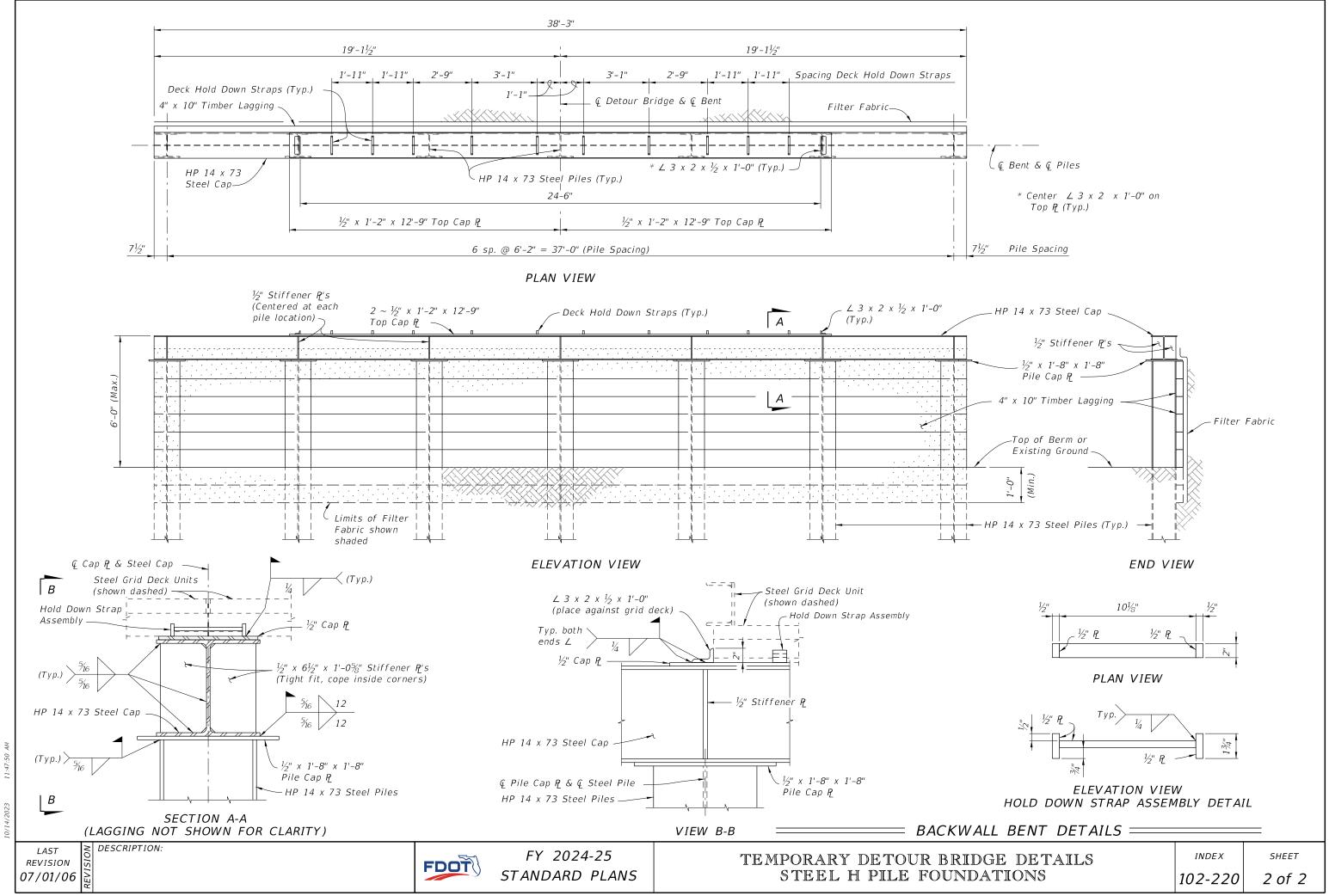
TABLE 7				
Bridge Roadway width (ft)	Transom Part #	(x) Q to inner truss to Q inner truss		
24	SC0017	26'-1"		
36	AB957	38'-4 ¹³ / ₁₆ "		
42	AB978	44'-43/8"		

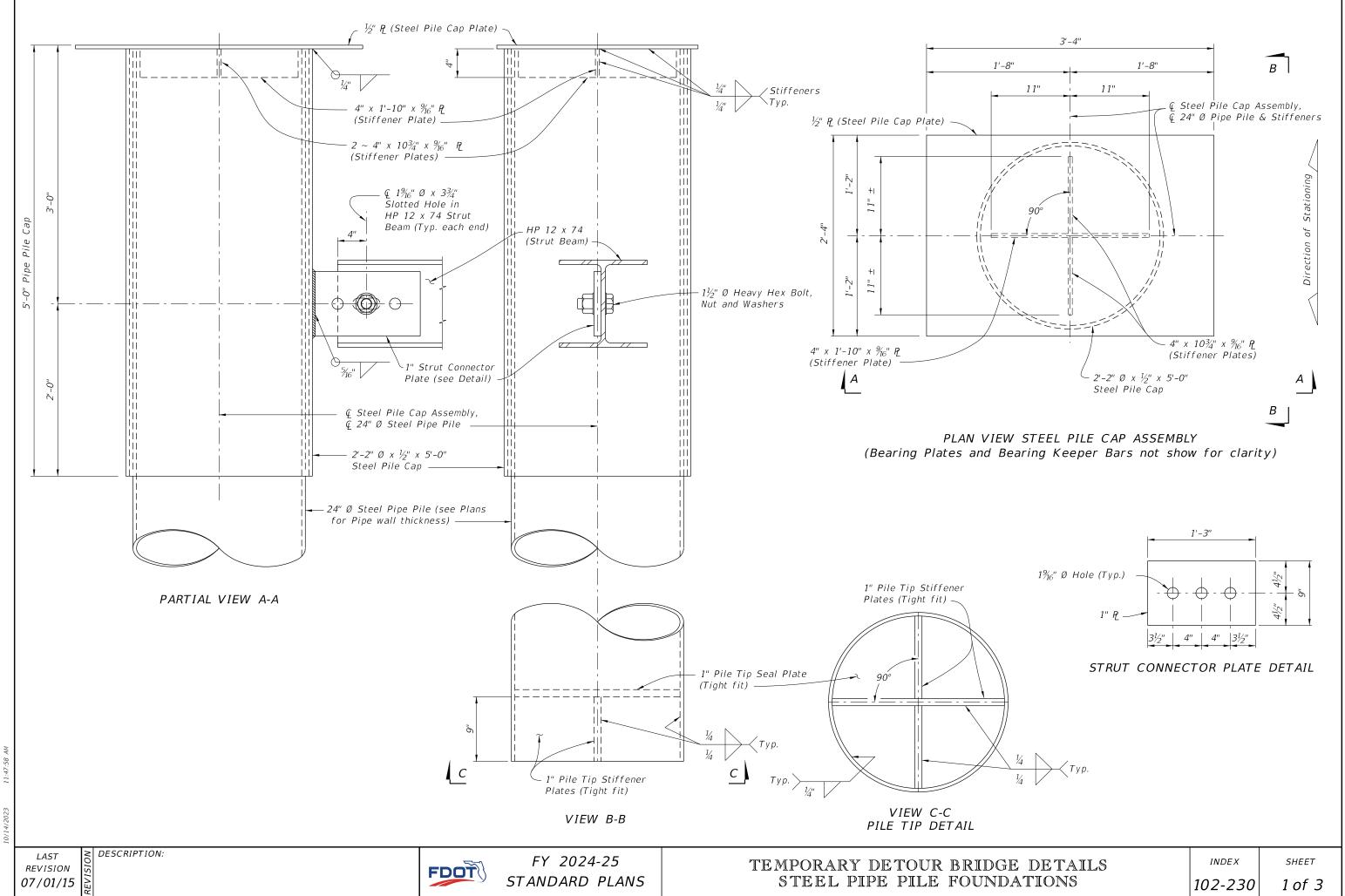


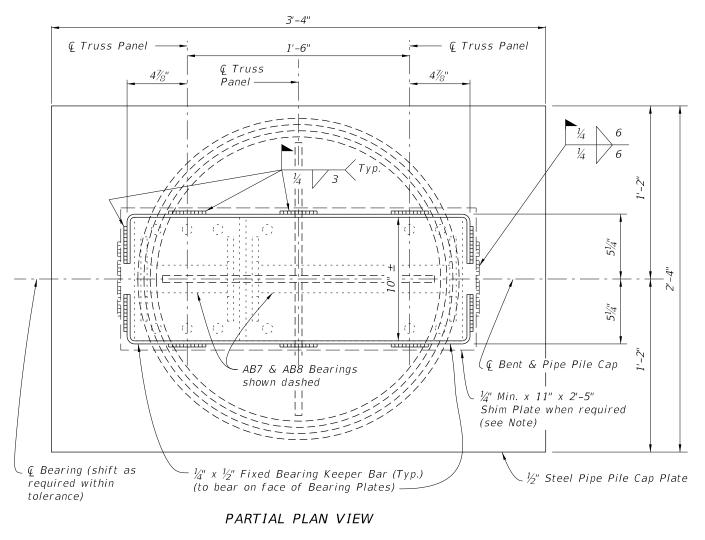


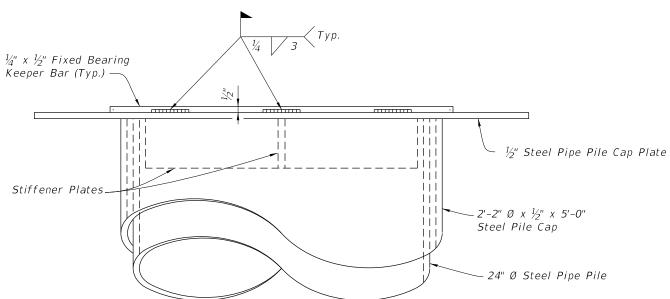




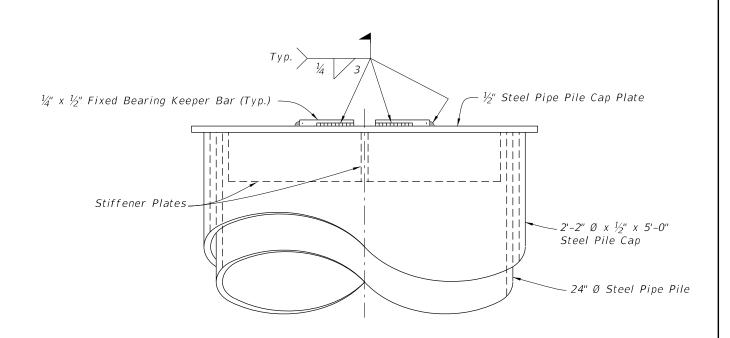








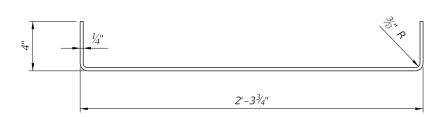
PARTIAL ELEVATION VIEW



END VIEW

Note:

Use Shim Plates as required to provide equal bearing seat elevations across the bent. Vary thickness of Shim Plate across the pile cap plate to provide a level bearing area in the transverse direction.



FIXED BEARING KEEPER BAR DETAIL

= FIXED BEARING DETAILS ==

REVISION 01/01/16

DESCRIPTION:

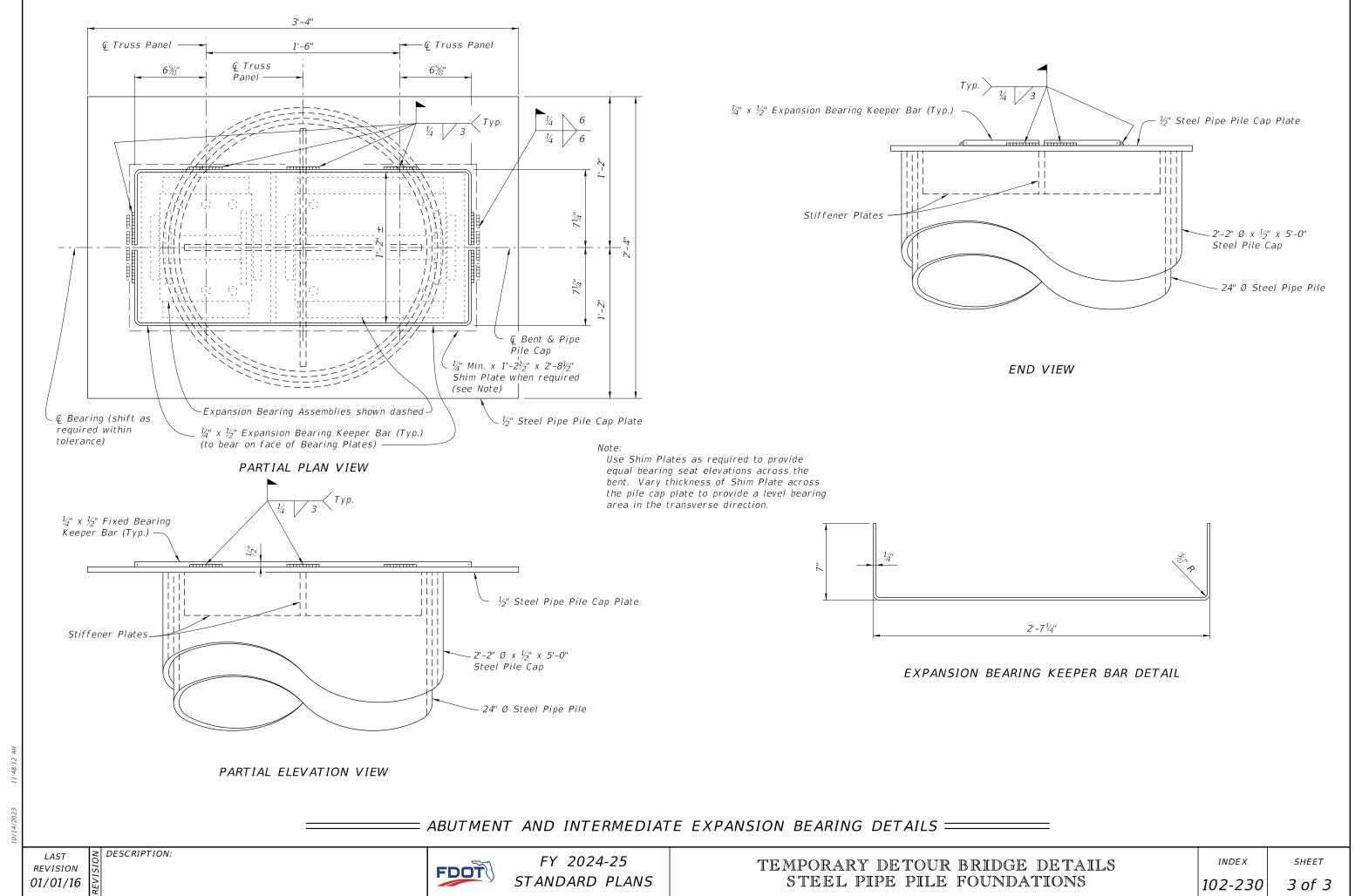
FY 2024-25 STANDARD PLANS

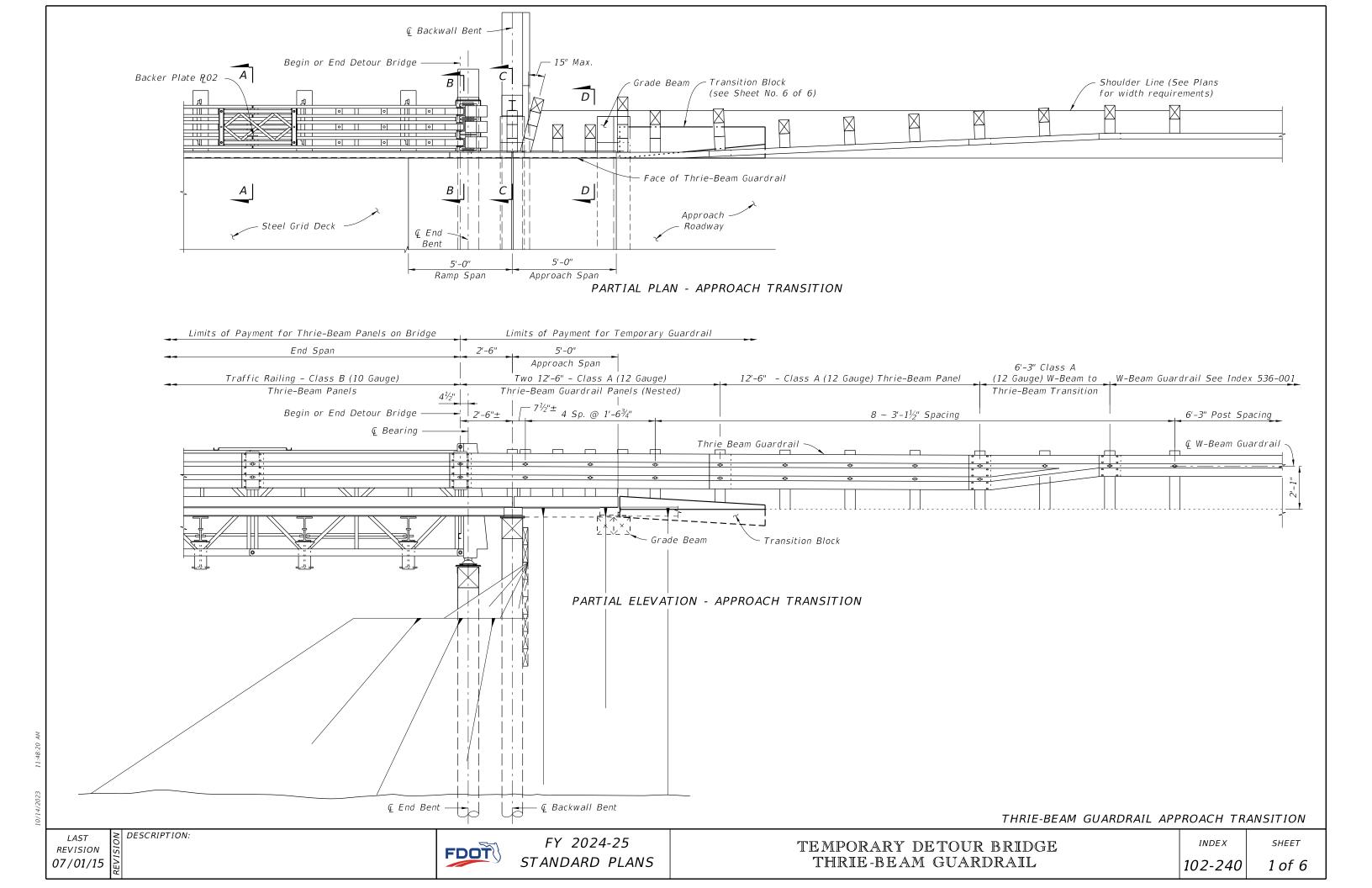
TEMPORARY DETOUR BRIDGE DETAILS STEEL PIPE PILE FOUNDATIONS

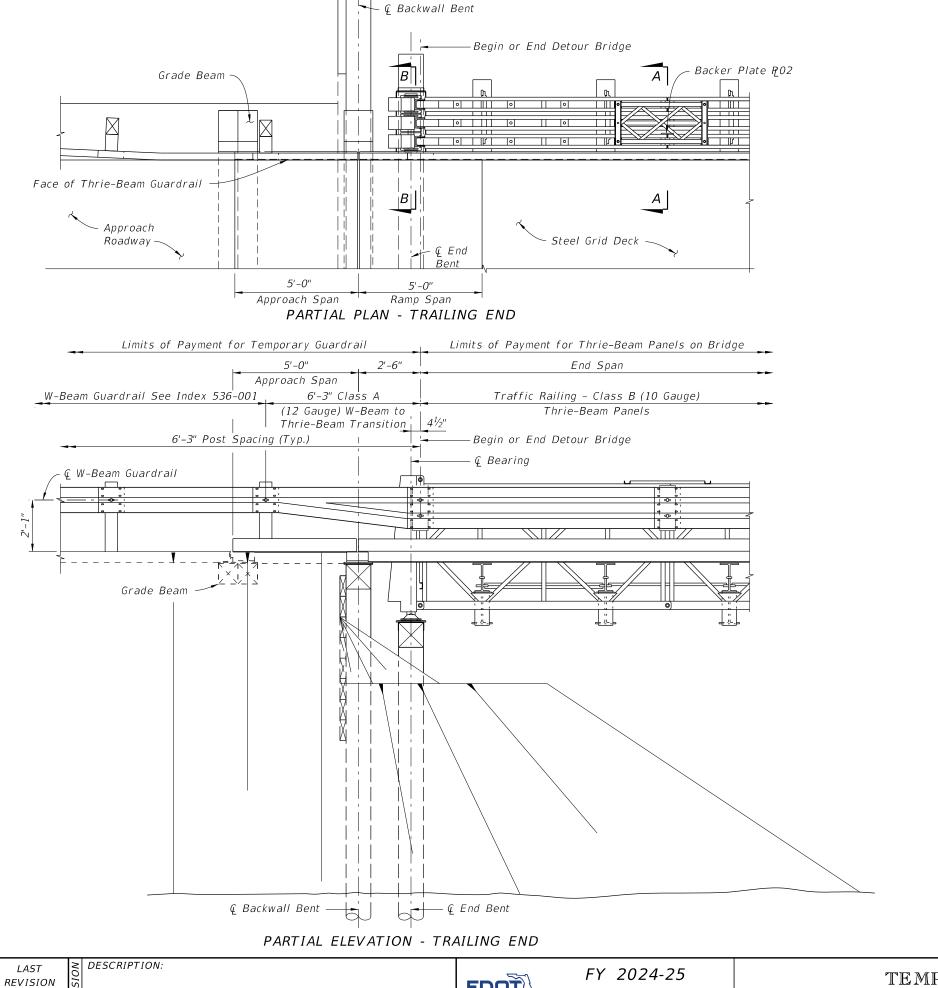
INDEX 102-230

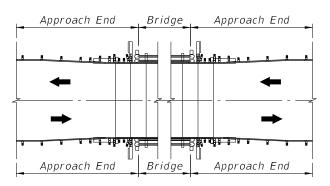
SHEET

2 of 3

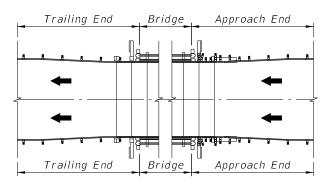








TWO-WAY TRAFFIC



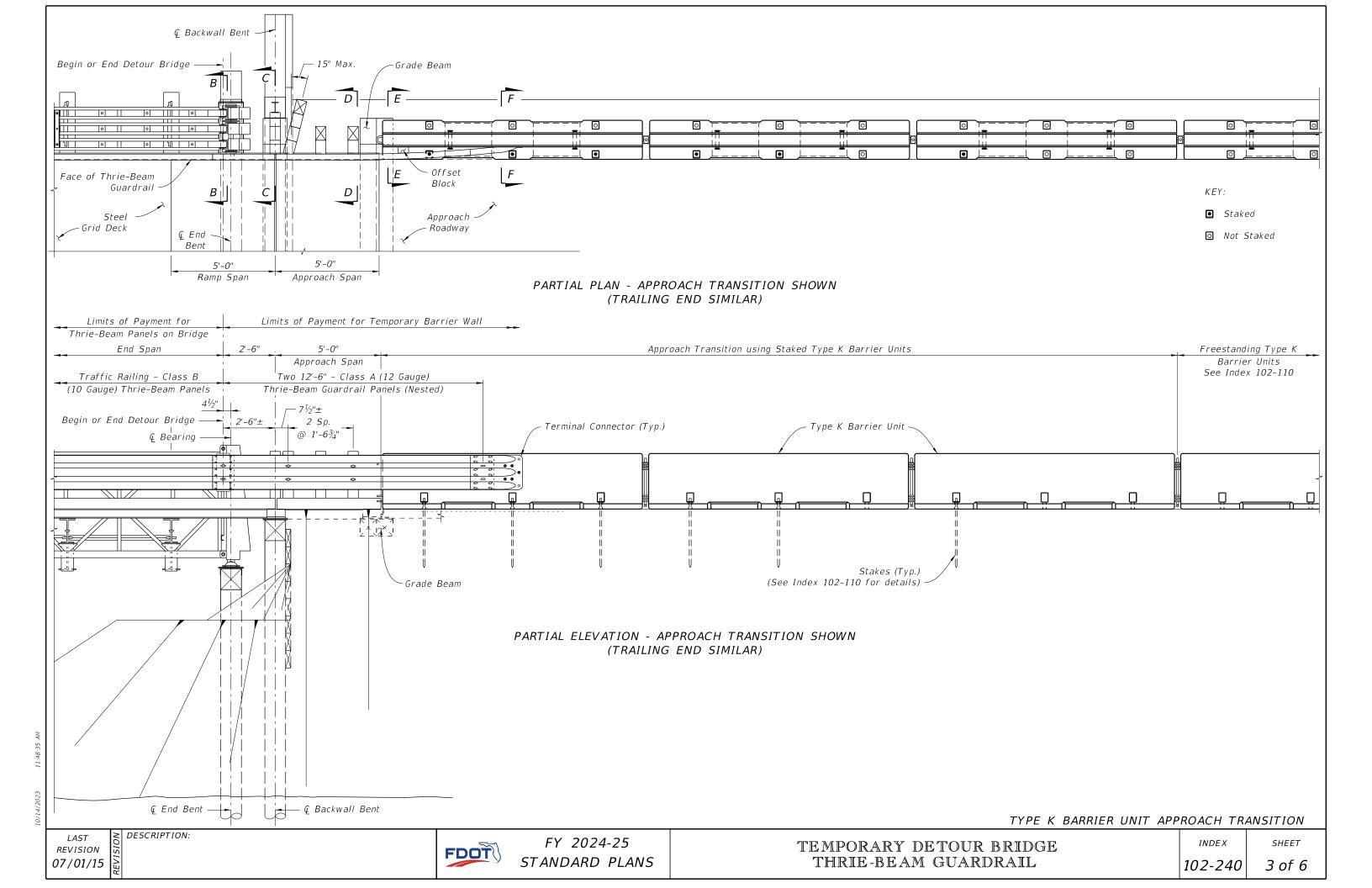
ONE-WAY TRAFFIC END TRANSITION APPLICATION DETAILS

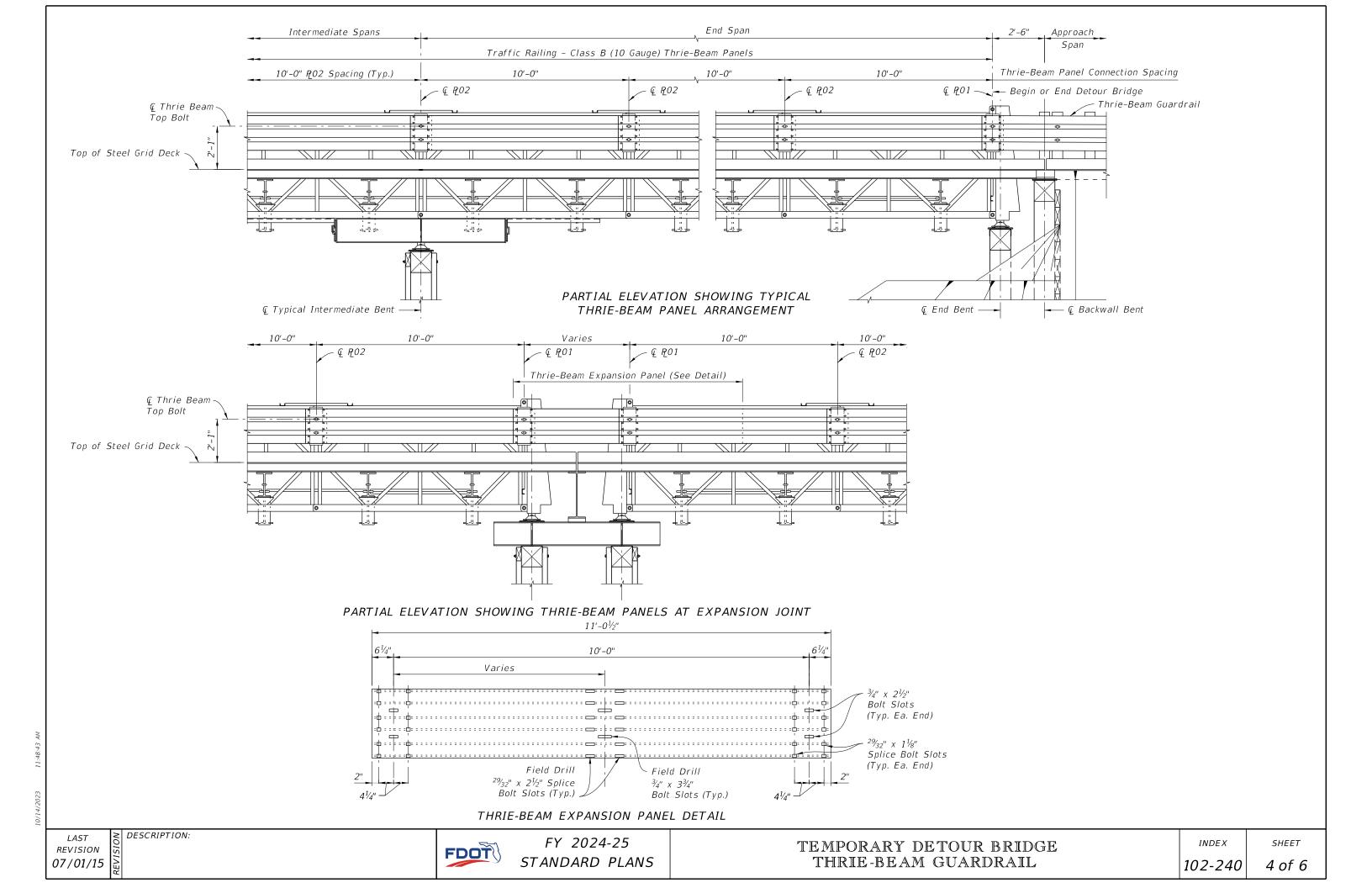
THRIE-BEAM GUARDRAIL TRAILING END TRANSITION

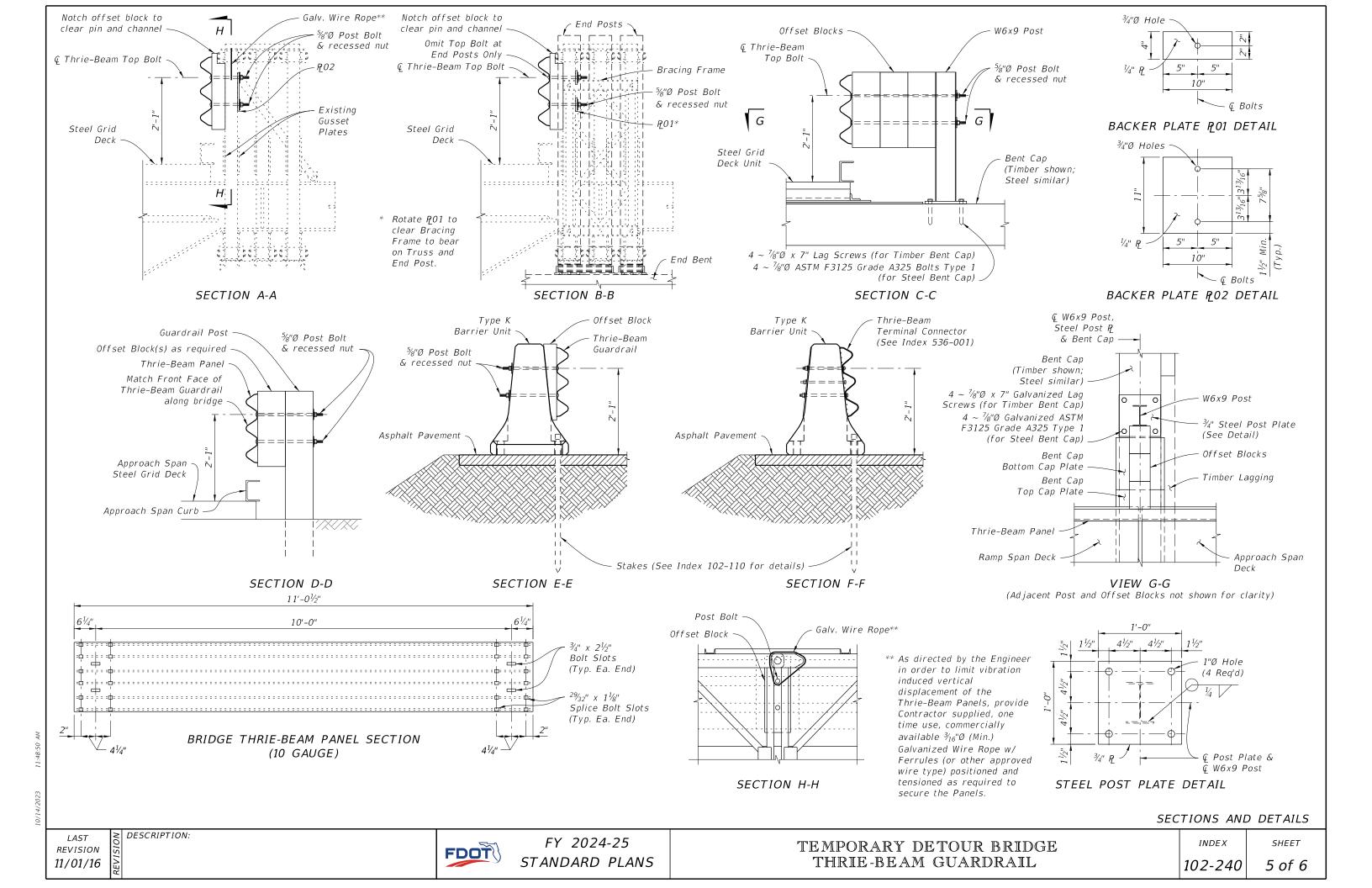
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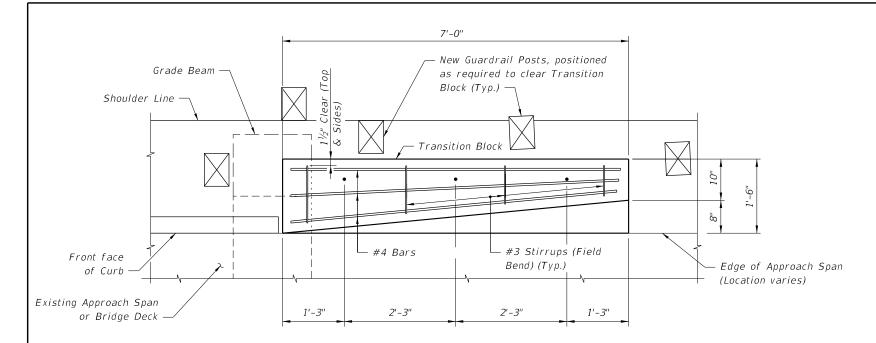
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FY 2024-25 STANDARD PLANS

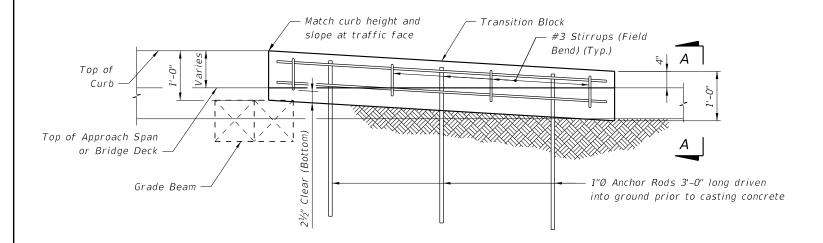






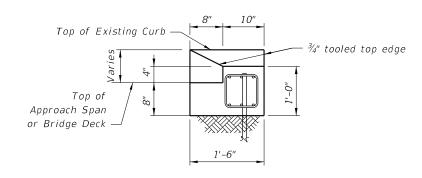


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

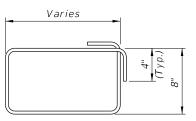


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES				
ITEM UNIT QUANTITY				
Concrete Class NS	CY	0.4		
Reinforcing Steel	LB	61		
Guardrail (Reset)	LF	12.5		



END VIEW A-A



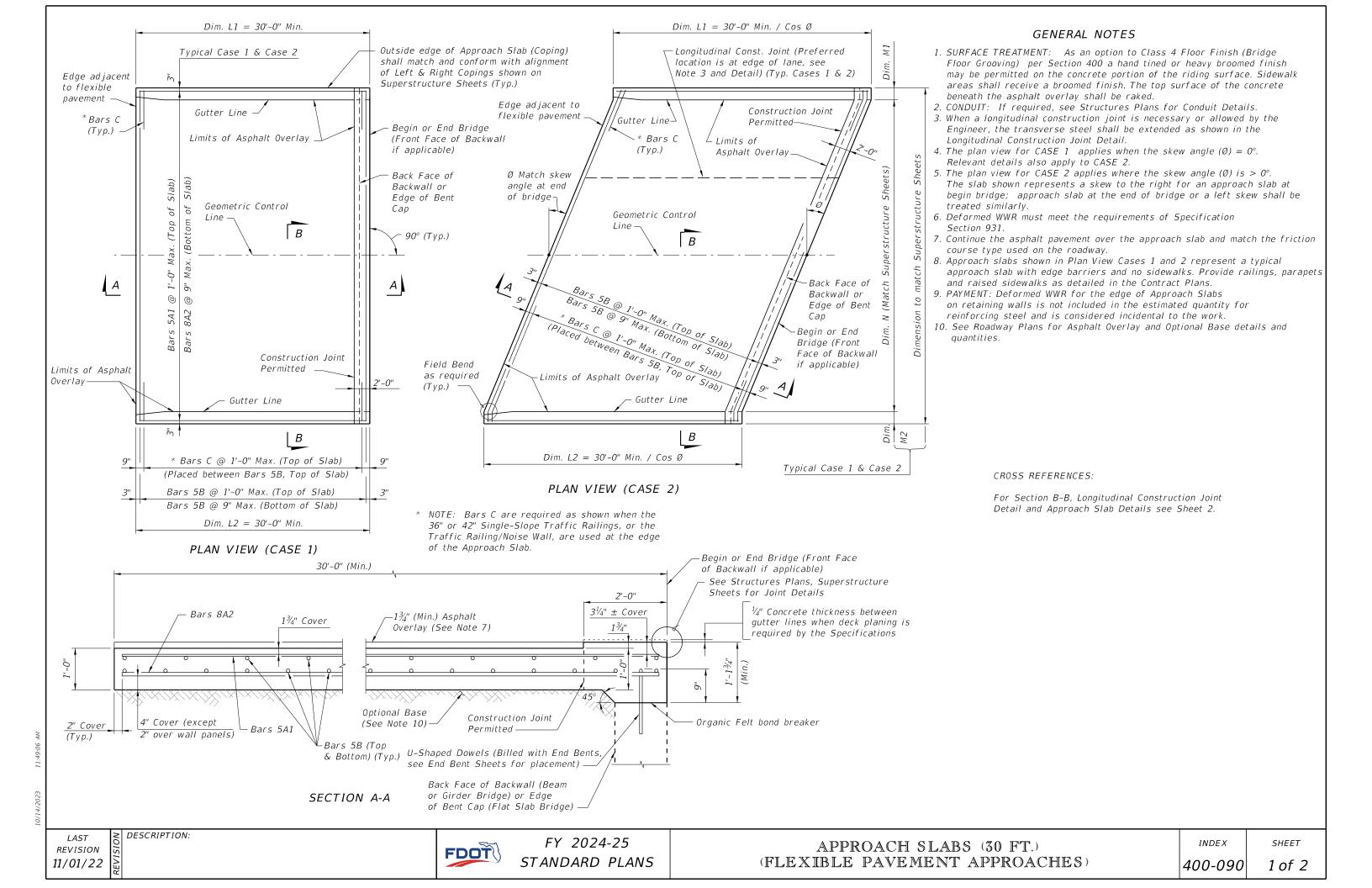
#3 STIRRUP (FIELD BEND)

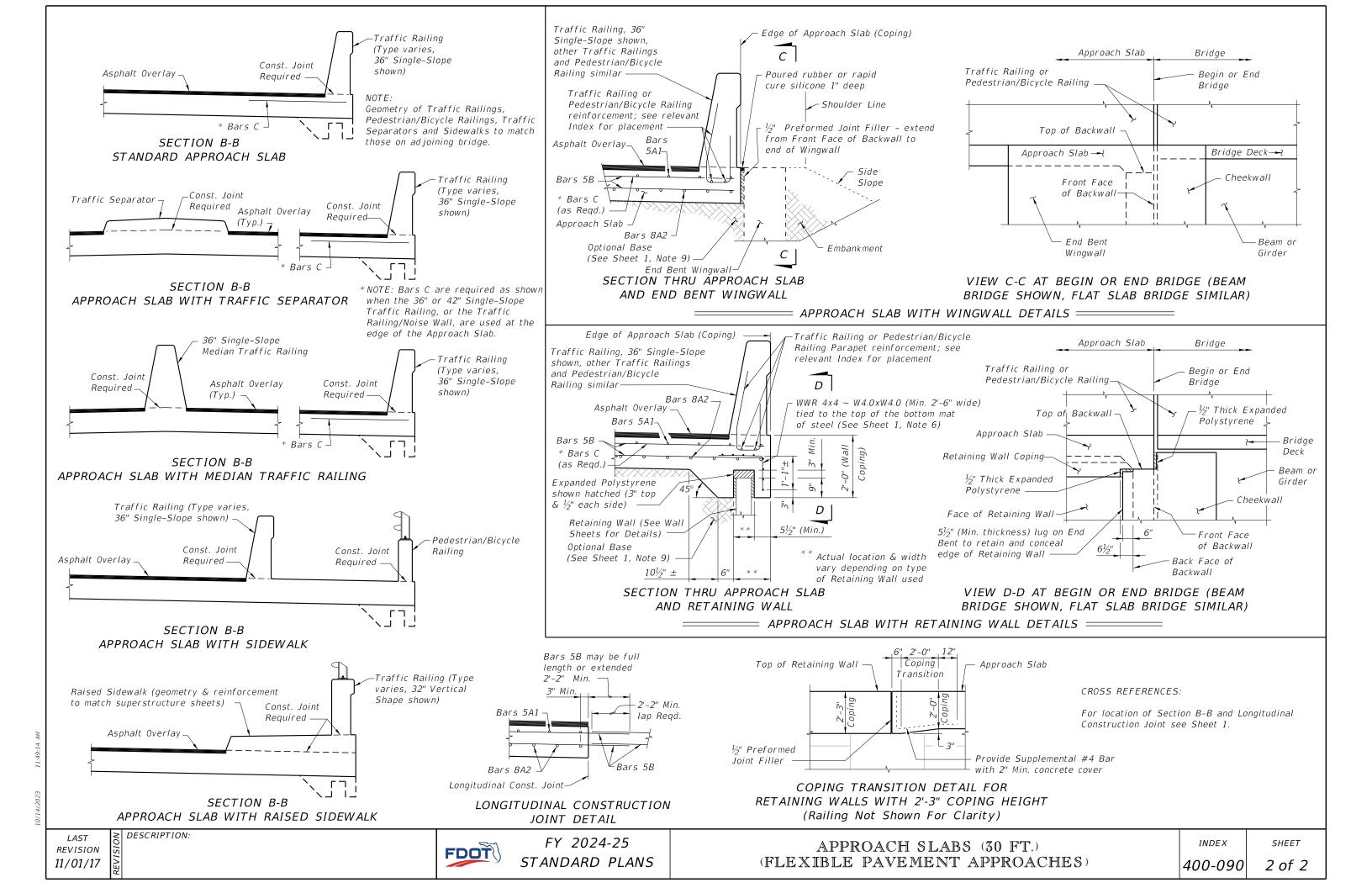
NOTES:

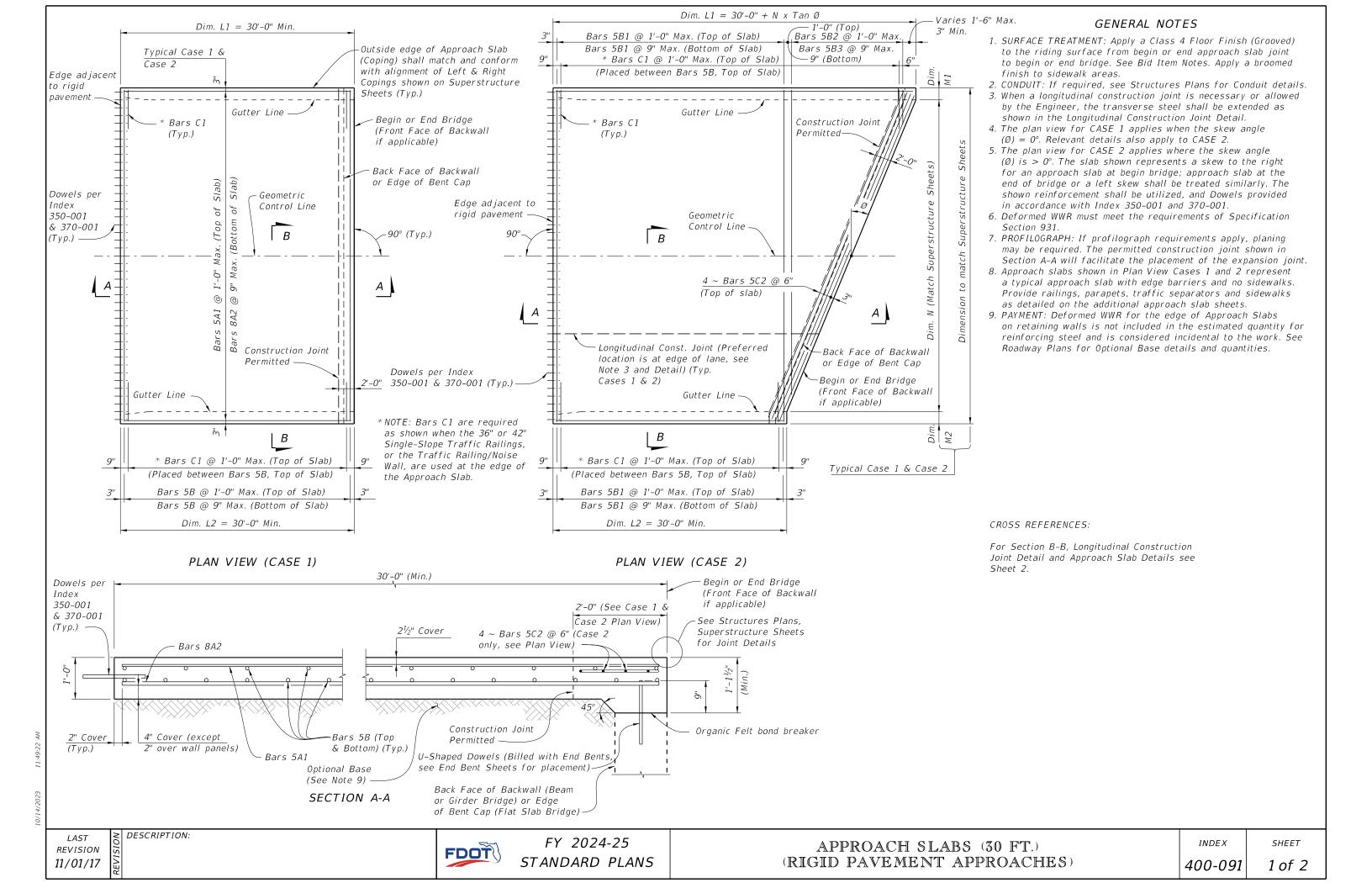
REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

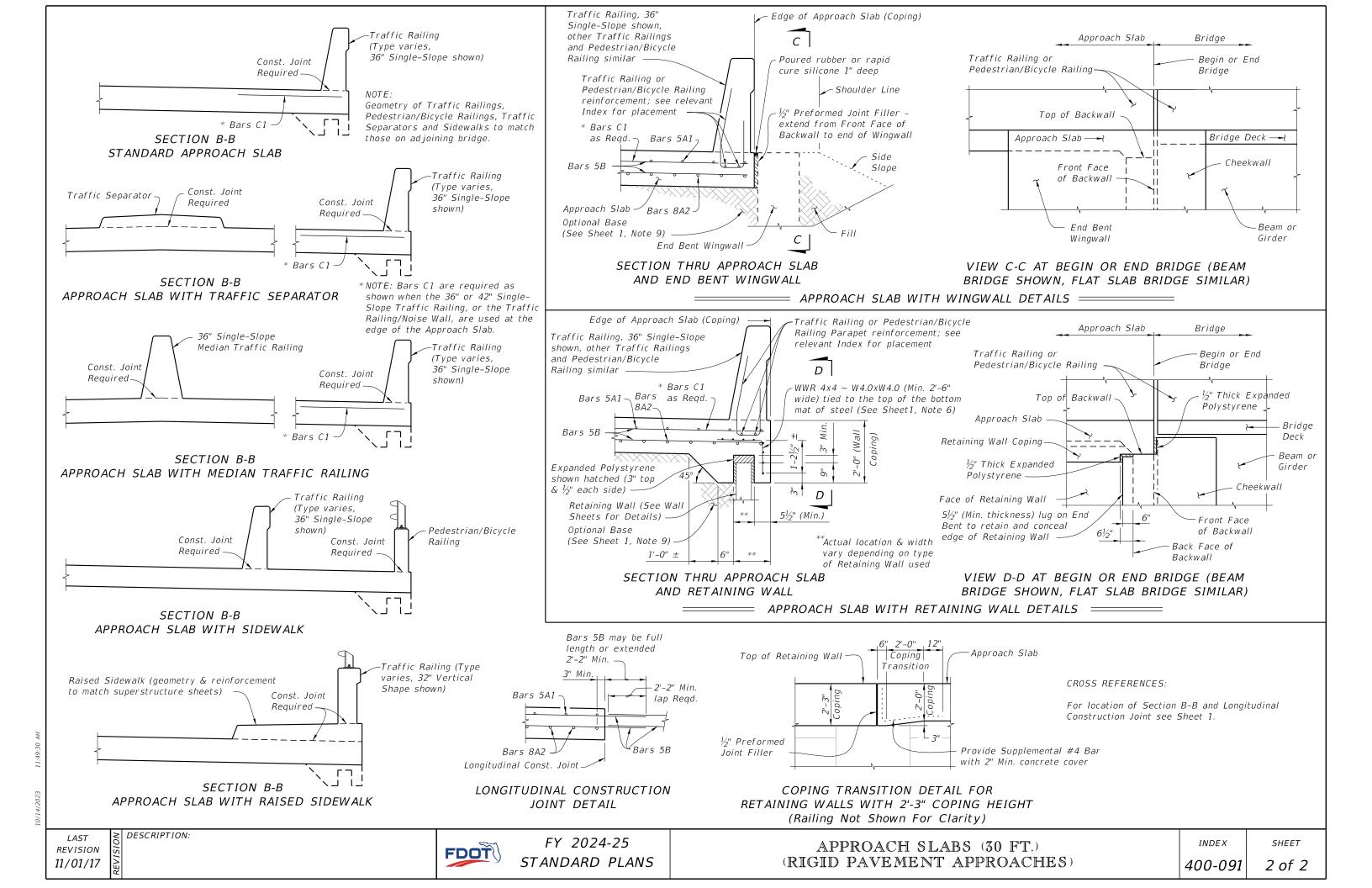
ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

DESCRIPTION:

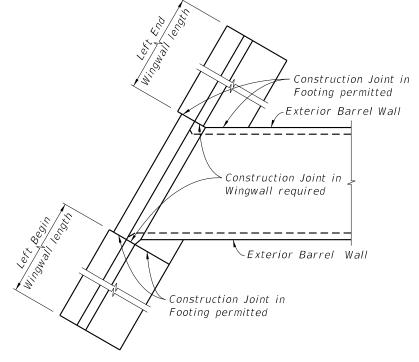








NOTE: All headwall and culvert skew angles are measured in degrees from a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic "B".



PART PLAN SHOWING PARALLEL WINGWALLS AND LOCATION OF CONSTRUCTION JOINTS

NOTE:

DESCRIPTION:

Construction Joints in wingwalls and footings are located as follows: For non-skewed wingwalls they are located adjacent to the exterior face of the exterior barrel wall; when the Q of wingwall and Q of exterior barrel wall results in an acute angle see Left End Wingwall above, and when the angle is obtuse see Left Begin Wingwall above and Detail C (Sheet 5).

GENERAL NOTES:

LIVE LOAD: HL-93.

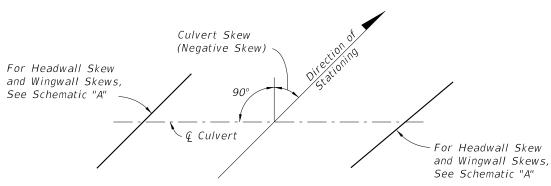
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 8.

REINFORCING STEEL: See the "Box Culvert Data Tables" in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.

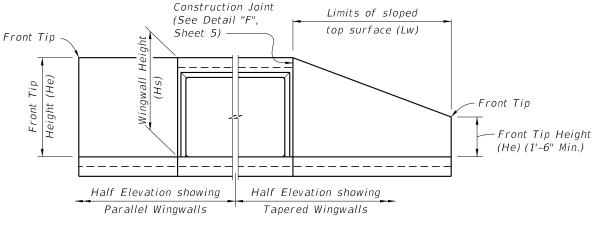


SCHEMATIC "B" - PLAN VIEW CULVERT ALIGNMENT

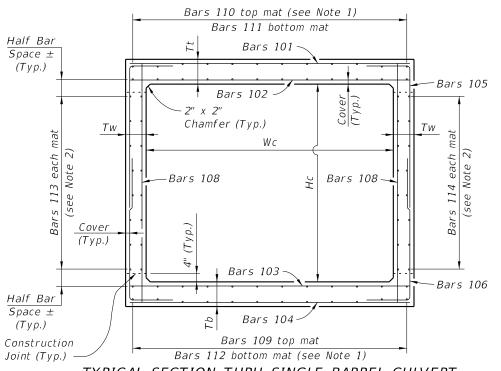
NOTE: For Culvert Skew see Contract Plans.

TAB	LE 1 - MI	NIMUM E	BAR SP	LICE LEN	GTHS
	FOR LON	GITUDIN.	AL REI	NFORCIN	G
BAR	SPLICE (CLASS B)	BAR	SPLICE (CLASS B)
SIZE	CLASS II	CLASS IV	SIZE	CLASS II	CLASS IV
	(3400 psi)	(5500 psi)		(3400 psi)	(5500 psi)
#3	1'-4"	1'-0"	#8	3'-5"	2'-8"
#4	1'-9"	1'-4"	#9	4'-3"	3'-4"
#5	2'-2"	1'-8"			
#6	2'-7"	2'-0"			
#7	3'-0"	2'-4"			

TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.



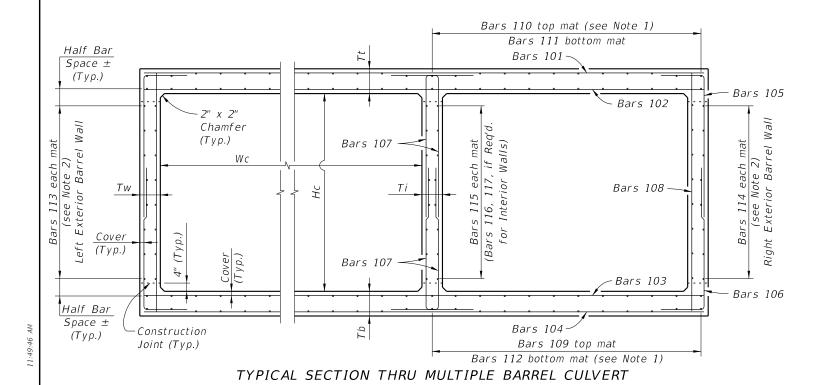
END ELEVATION OF CULVERT

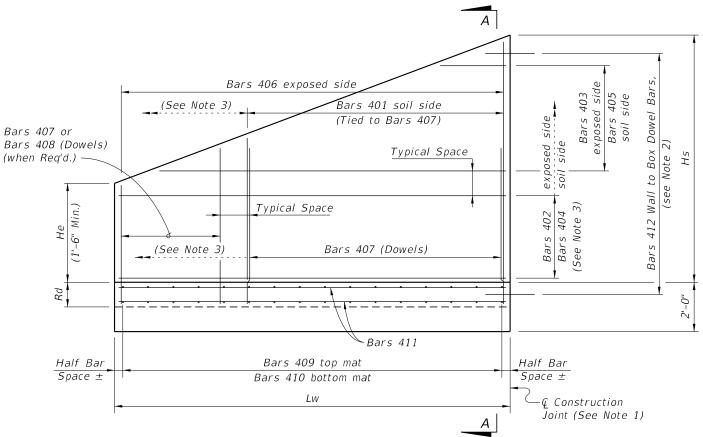


TYPICAL SECTION THRU SINGLE BARREL CULVERT

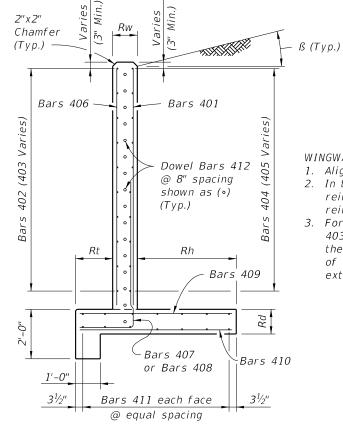
CULVERT BARREL NOTES:

- 1. Space Bars 110 and 112 with a bar in each corner, and at the ${\mathbb Q}$ of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4" clear.





WINGWALL ELEVATION - Variable Height (Left End shown - other corners similar)



WINGWALL NOTES:

- 1. Align construction joint perpendicular to wingwall.
- 2. In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

WINGWALL SECTION A-A

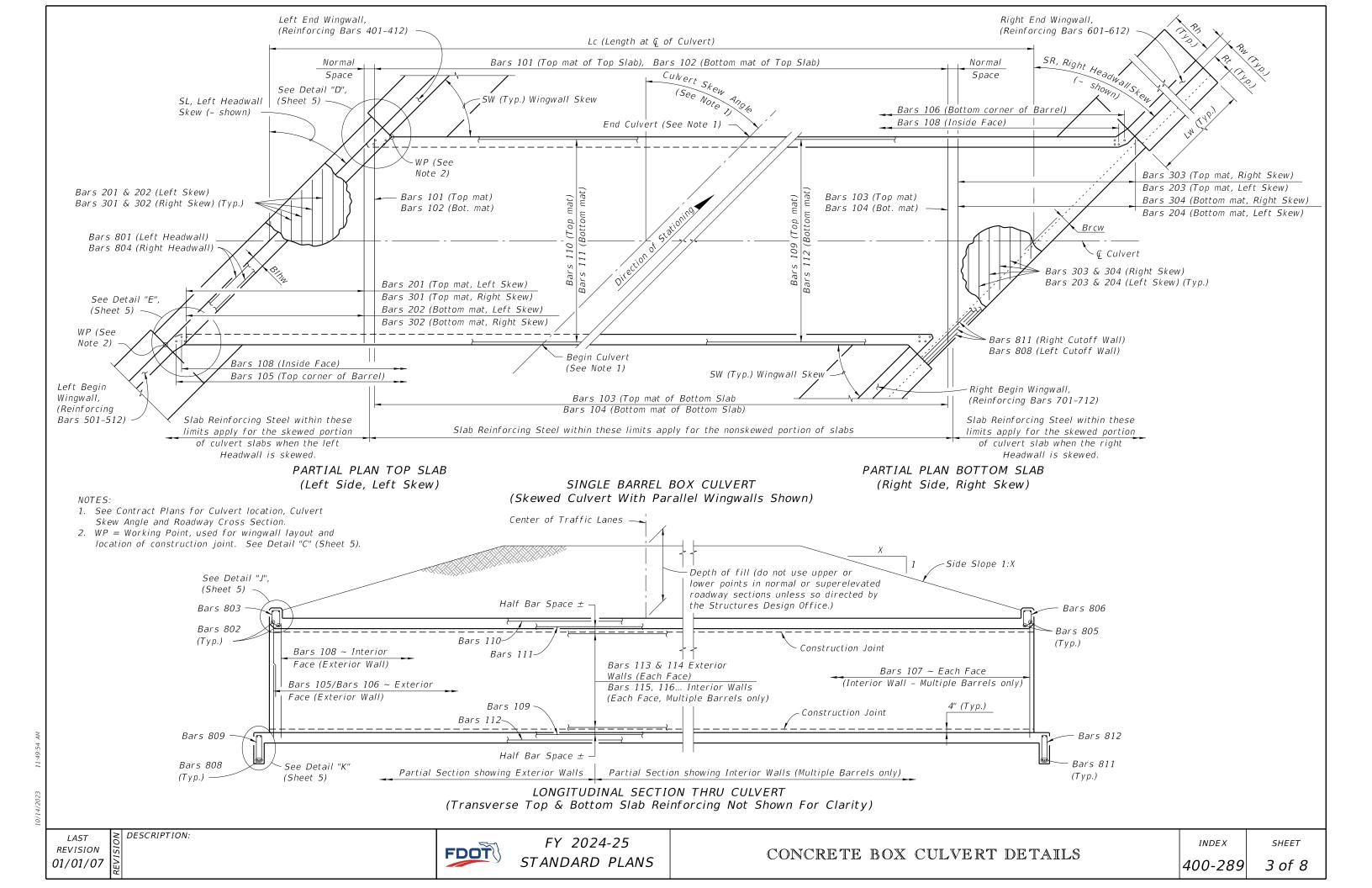
REVISION 07/01/13

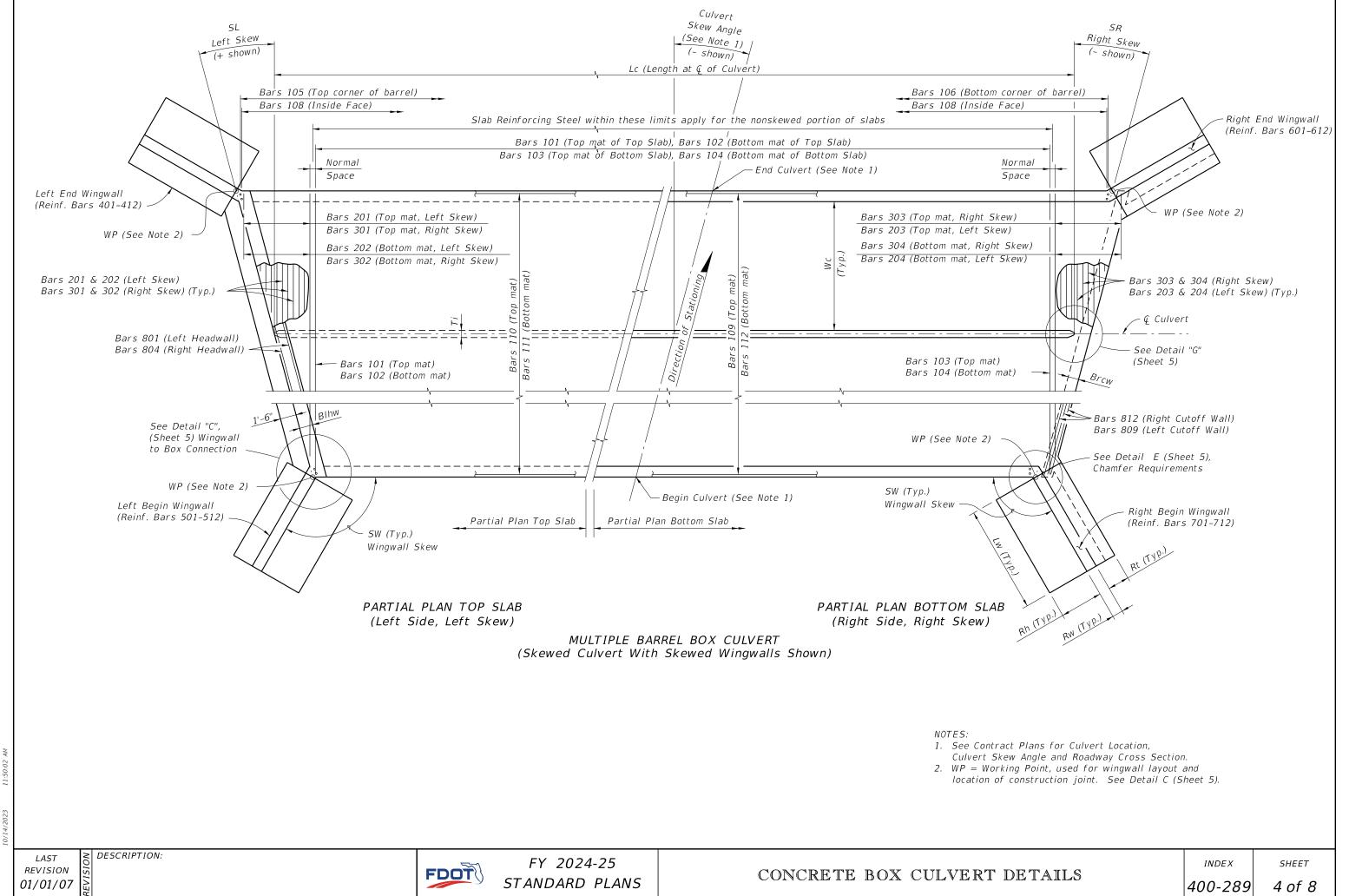
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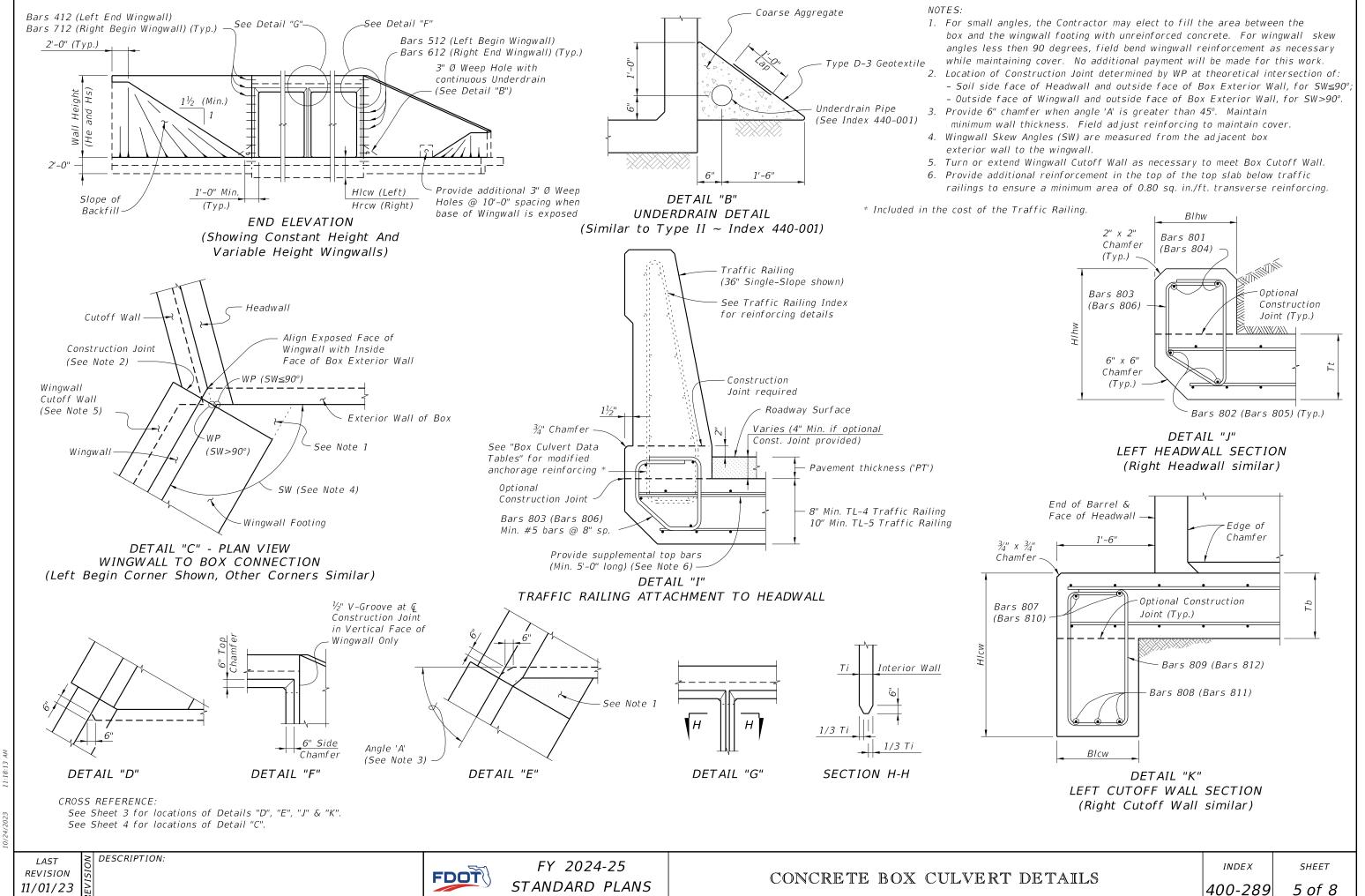
FDOT

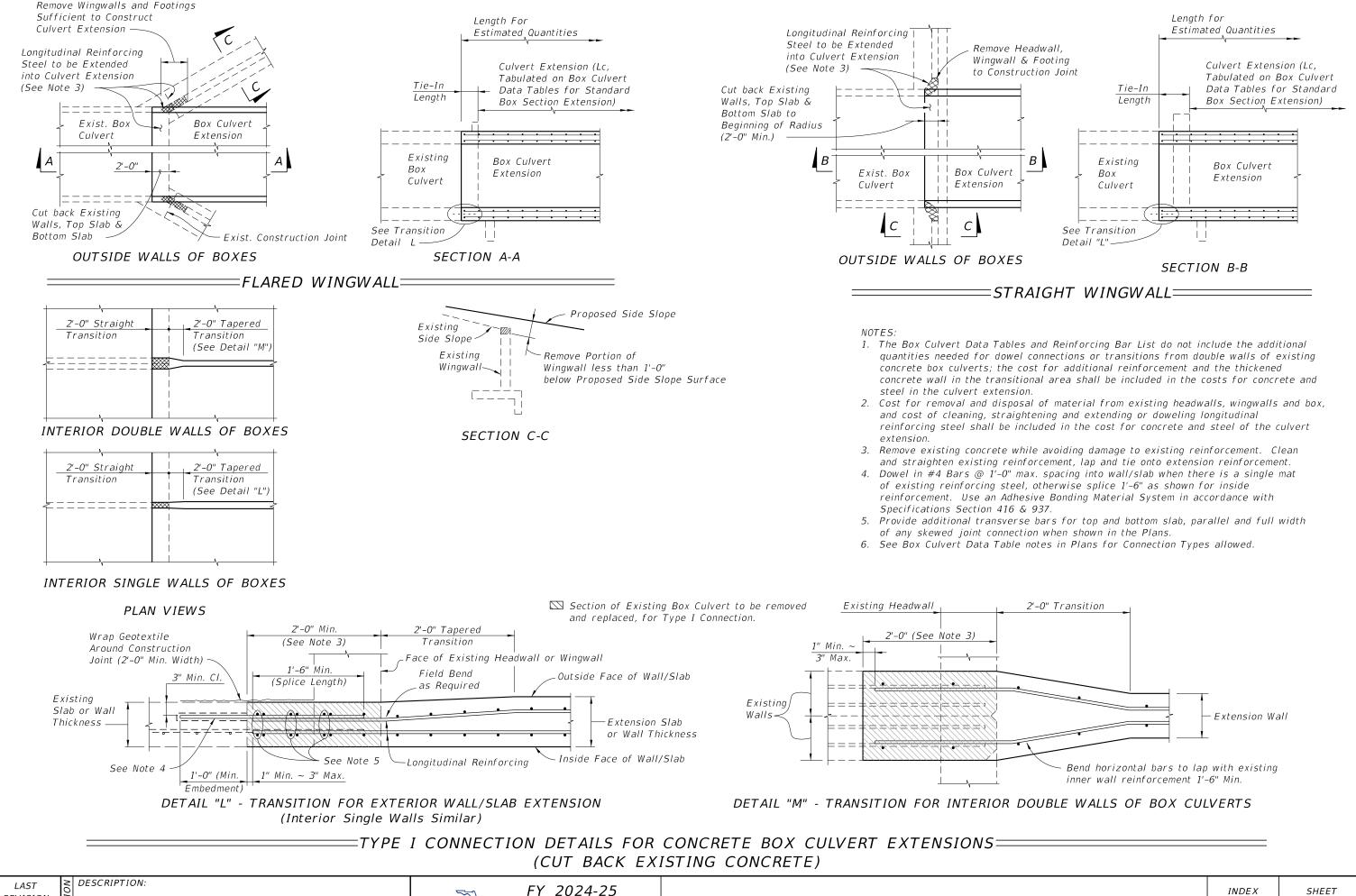
FY 2024-25 STANDARD PLANS INDEX

SHEET









REVISION 11/01/23

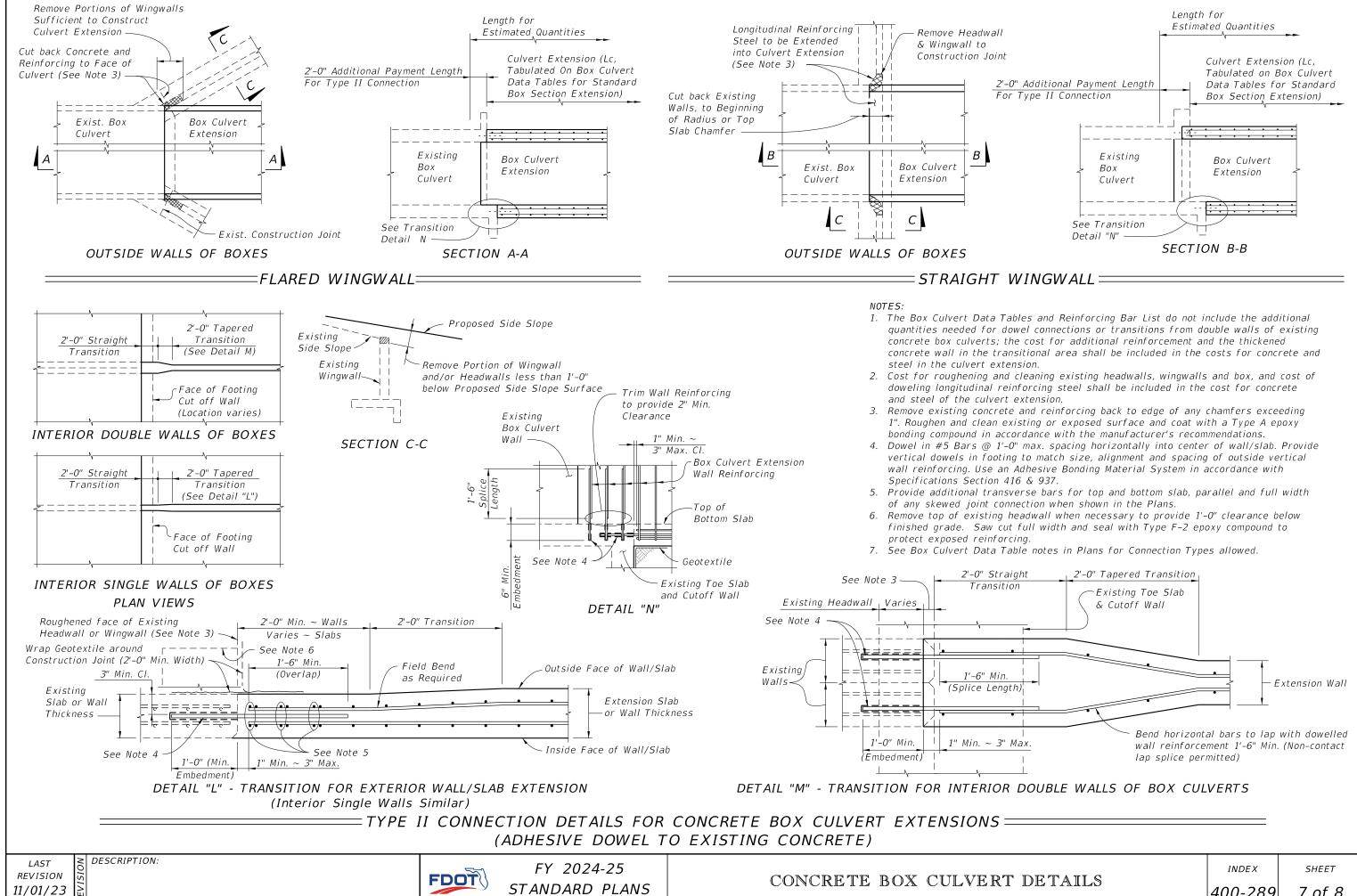
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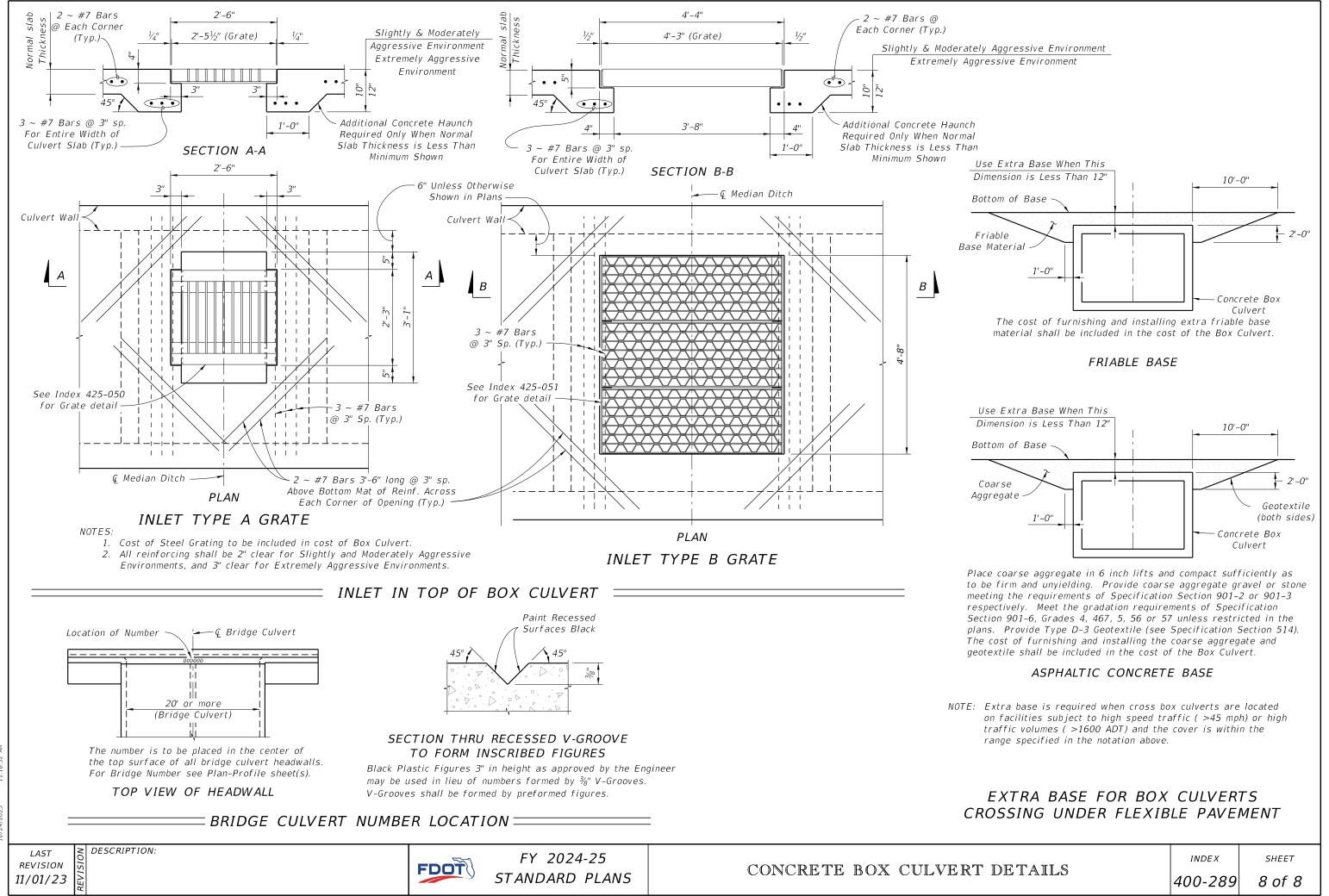
FY 2024-25 STANDARD PLANS

CONCRETE BOX CULVERT DETAILS

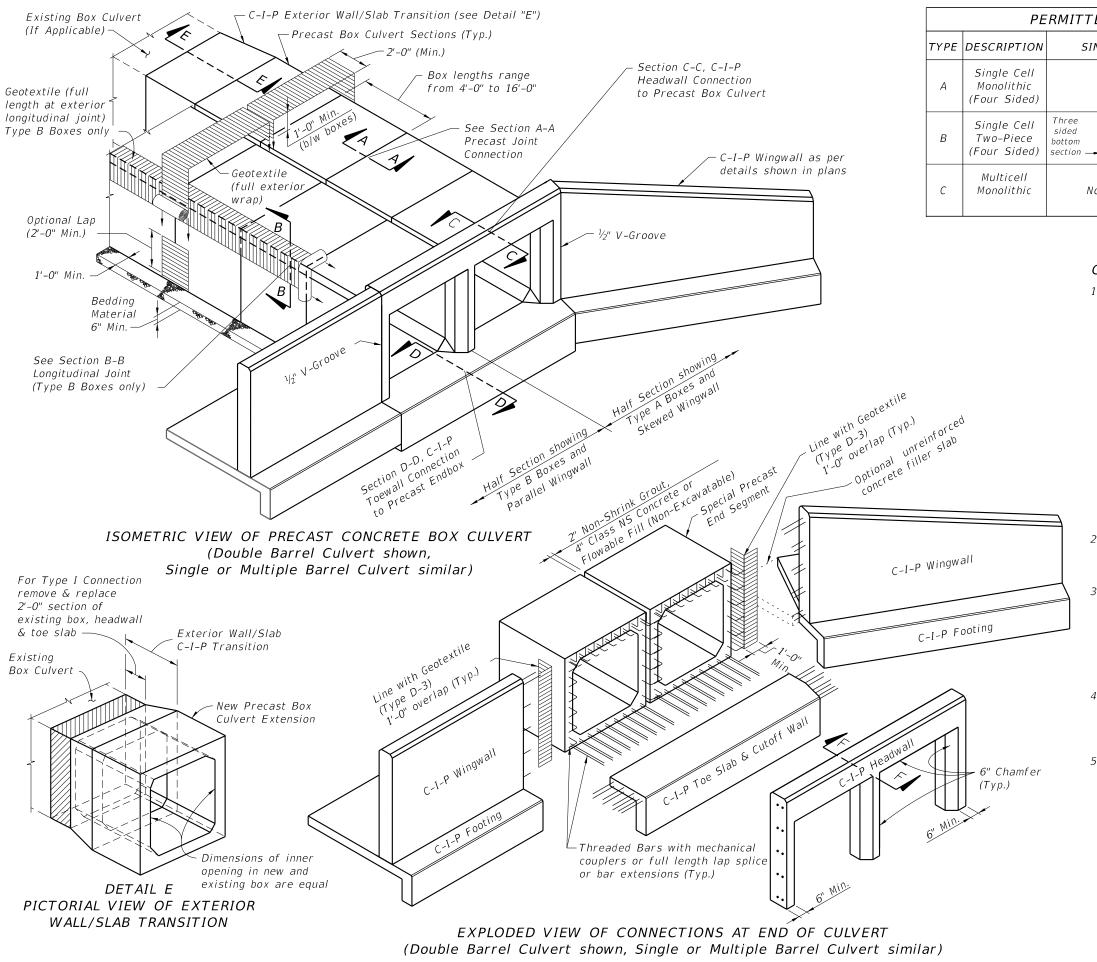
INDEX 400-289

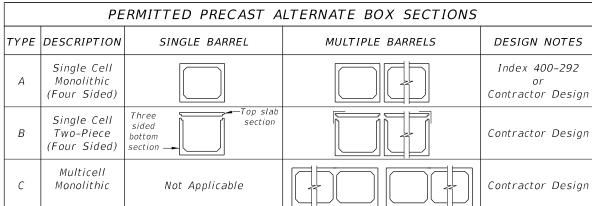
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1 5505/85/01





GENERAL NOTES:

1. Specifications:

General:

FDOT Standard Specifications for Road and Bridge Construction, Section 410 (current edition, and supplements thereto).

Concrete (Precast): Class III or Class II Modified (5,000 psi) for slightly aggressive environments.

Class IV (5,500 psi) for moderately to extremely aggressive environments.

Concrete (Cast-In-Place):

Class II (3,400 psi) for slightly aggressive environments. Class IV (5,500 psi) for moderately to extremely aggressive environments.

Reinforcing Steel:

Maintain minimum clearance of 2" for slightly and moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. Equal area substitution of welded wire (WWR) reinforcement is permitted.

- 2. Work this Index with the Cast-In-Place Concrete Box Culvert Details and Data Tables shown in the plans, Index 400-289 and the Precast Concrete Box Culverts shown in the shop drawings.
- 3. All joints between precast sections must be tongue & groove with joint sealant. Joints between cast-in-place & precast sections shall have longitudinal reinforcing extending from top, bottom & both side slabs of the precast box tied to the cast-in-place reinforcement. Single barrel culverts may have precast headwalls cast integrally with the end segment when approved by the Engineer.
- 4. Extension of existing multiple barrel box culverts with multiple single cell precast box culverts is not permitted unless approved by the District Structures Engineer. Full transition details must be shown in the shop drawings when approved.
- 5. Culverts larger than the specified size may be substituted with no additional payment to the Contractor. Substitution must be approved by the Engineer, minimum earth cover and invert elevations shown in the Contract Documents must be maintained.

REVISION 11/01/23

DESCRIPTION:

FDOT

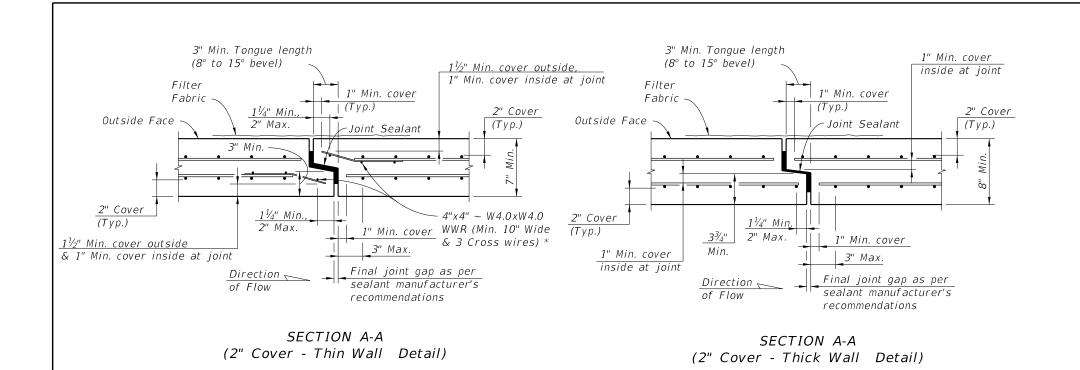
FY 2024-25 STANDARD PLANS

PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

INDEX

SHEET 1 of 5

400-291



3" Min. *∽Joint Sealant* Provide WWR or extend reinforcing into tongue (See Section A-A) ALTERNATE BOTTOM SLAB TRANSVERSE JOINT TYPICAL SECTION

(DOUBLE-SIDED TONGUE & GROOVE JOINT)

(All reinforcing not shown for clarity)

3" Min. Tongue length

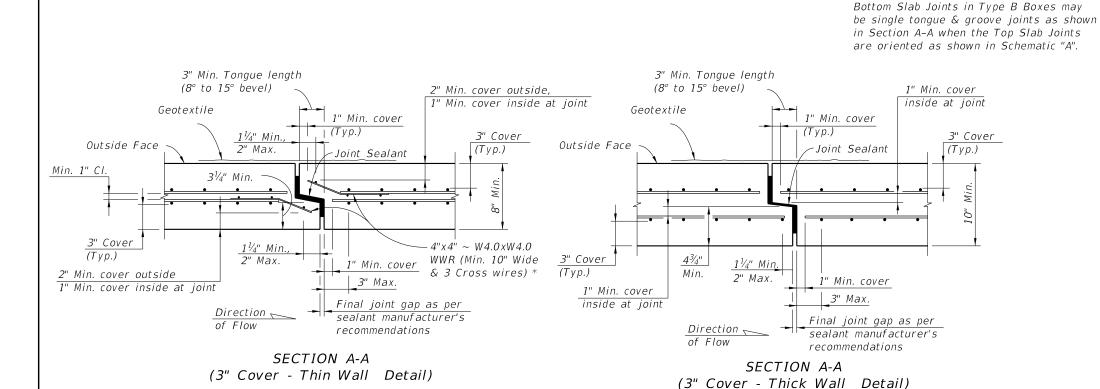
(8° to 15° bevel)

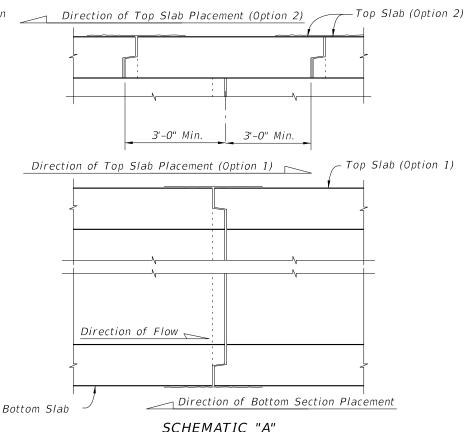
3" Min.

See Section A-A

cover requirements

for reinforcing





TYPE B BOX SECTION PLACEMENT FOR SINGLE TONGUE & GROOVE JOINTS

= TWO-PIECE PRECAST SEGMENT ADDITIONAL JOINT DETAILS (TYPE B BOX)

PRECAST SEGMENT TO SEGMENT TONGUE & GROOVE TRANSVERSE JOINTS =

* At the Contractor's option when the box culvert

reinforcing utilizes WWR, extend wall and slab

reinforcing into the joint and bend to maintain

of box to allow bending of the WWR.

cover in lieu of 4"x4" ~ W4.0xW4.0 WWR at joint. Transverse wire in tongue may be cut at corners

REVISION 11/01/23

DESCRIPTION:

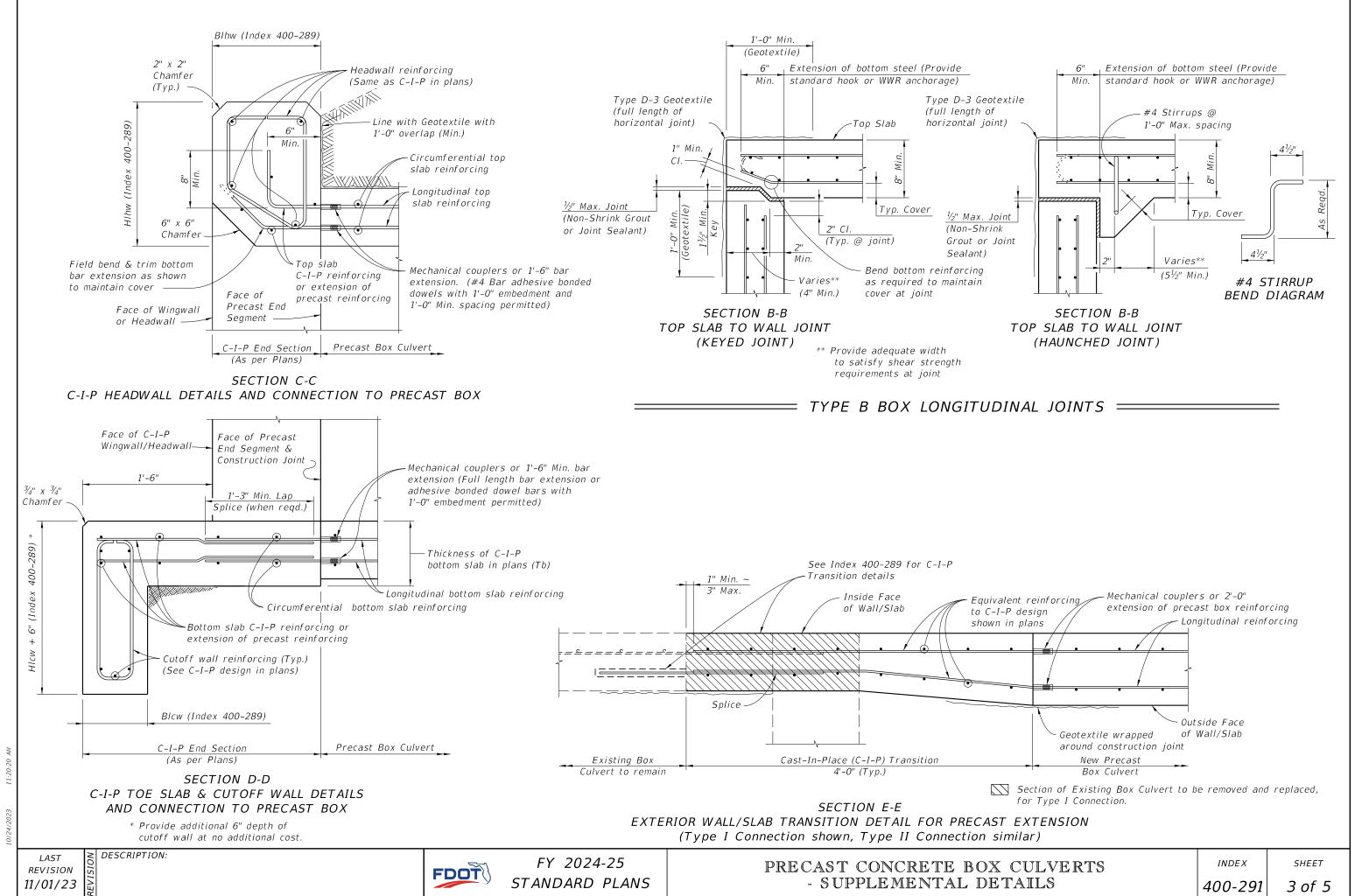
FDOT

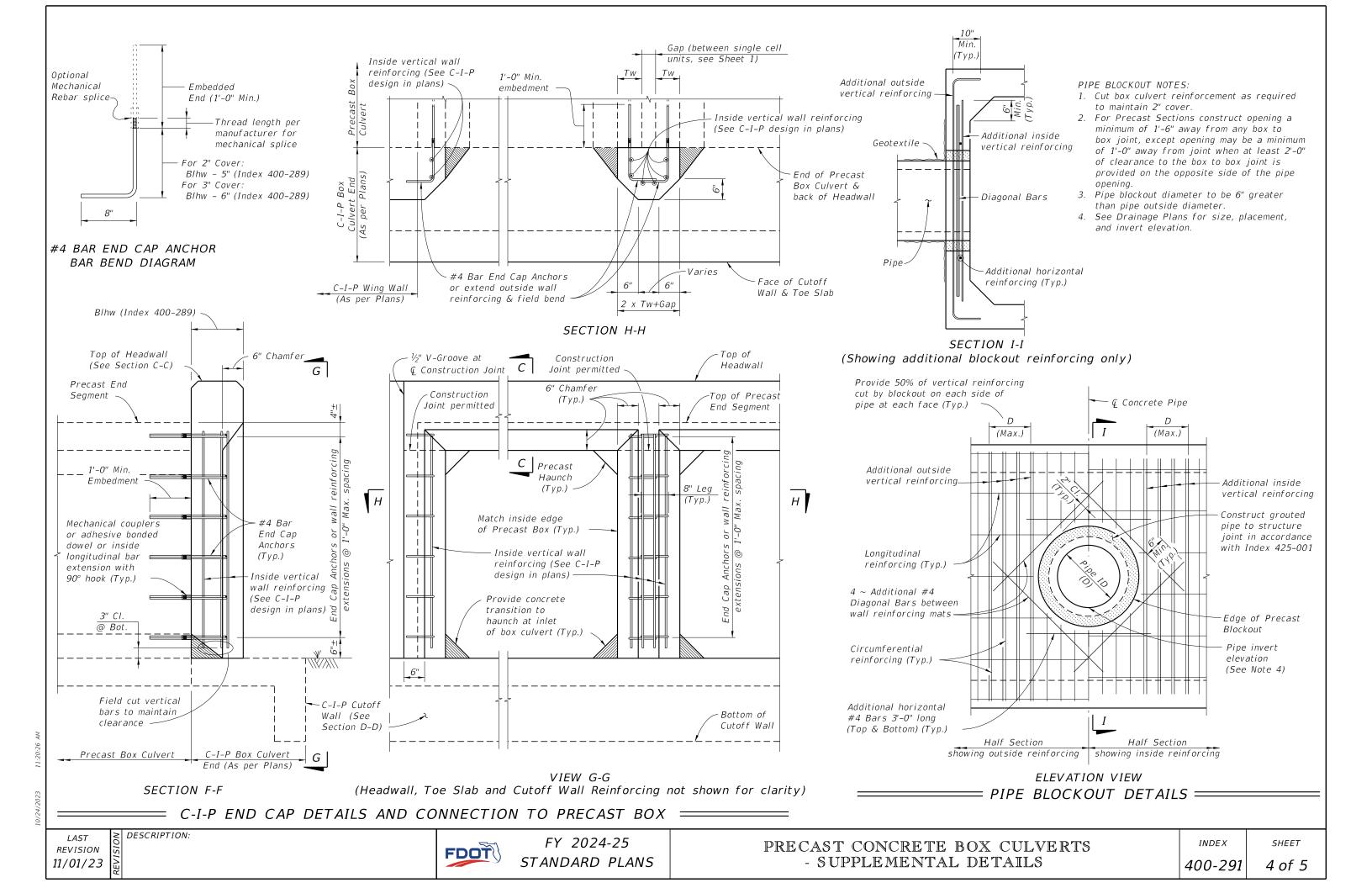
FY 2024-25 STANDARD PLANS PRECAST CONCRETE BOX CULVERTS

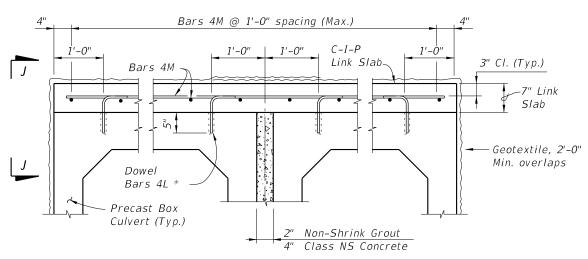
INDEX

SHEET

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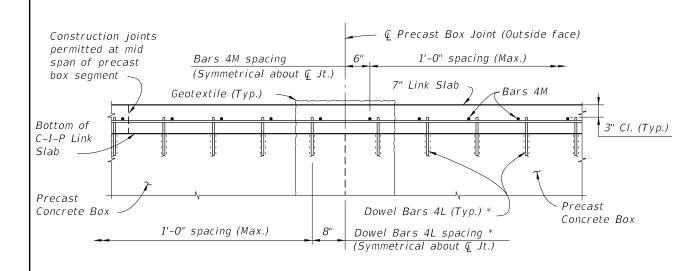






LINK SLAB TYPICAL SECTION (Multiple Barrel Culvert shown, Single Barrel Culvert similar)

* Install dowels with an Adhesive Bonding Material System in accordance with Specification Section 416. The Contractor may substitute mechanical couplers in lieu of adhesive bonded dowels. Shift dowels to clear box culvert reinforcing.



VIEW J-J

LINK SLAB NOTES:

1. Provide a Cast-In-Place Link Slab to ensure uniform joint opening of precast box culverts when the differential settlement shown in the plans exceeds the following limits, except that a Link Slab is not required for differential settlements less than 1/2".

$$\Delta Y \leq \frac{(L)^2}{760 \times R \times W}$$

Where:

 $\Delta Y = Maximum Long-Term Differential Settlement (ft.)$

R = Exterior height of Box Culvert (ft.)

W = Length of Box Culvert Segments (ft.)

L = Effective length for single curvature deflection (ft.)

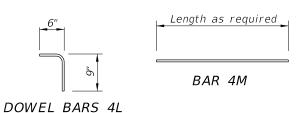
2. Extend Link Slab to back face of headwalls and to limits of existing box culverts for extensions.

ESTIMATED LINK SLA	B QUANTI	TIES
ITEM	UNIT	QUANTITY
Class II or IV Concrete (Culvert)	CY/SF	0.0216
Reinforcing Steel (Roadway)	Lb./SF	1.52

NOTE: Estimated quantities are based the plan area of precast box slabs, and are provided for information only. No additional payment will be made for Link Slabs where these are required for the precast box culverts.

	BILL OF RE	INFORCING STEE	L
MARK	SIZE	NO. REQ'D	LENGTH
L	4	2 per Barrel/Ft.	1'-3"
М	4	As Reqd.	As Reqd.

REINFORCING STEEL BENDING DIAGRAMS

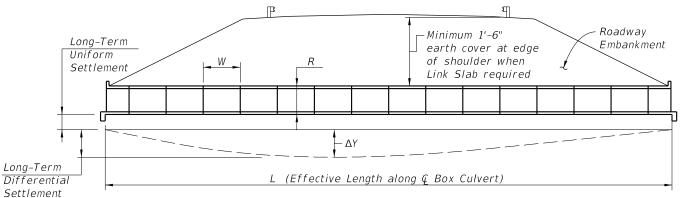


NOTES:

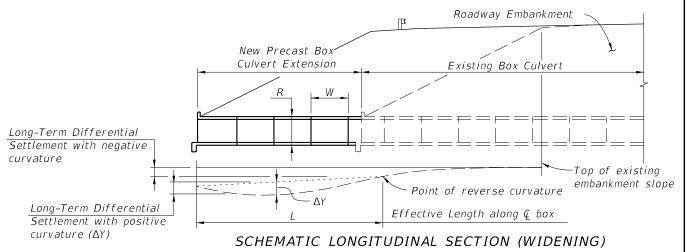
- 1. All bar dimensions are out to out.
- 2. Lap splice length for Bars 4M is 1'-4" minimum.

DESIGN NOTE:

1. Link Slab required when joint openings from differential settlement exceed 1/8" as determined in Link Slab Note 1.



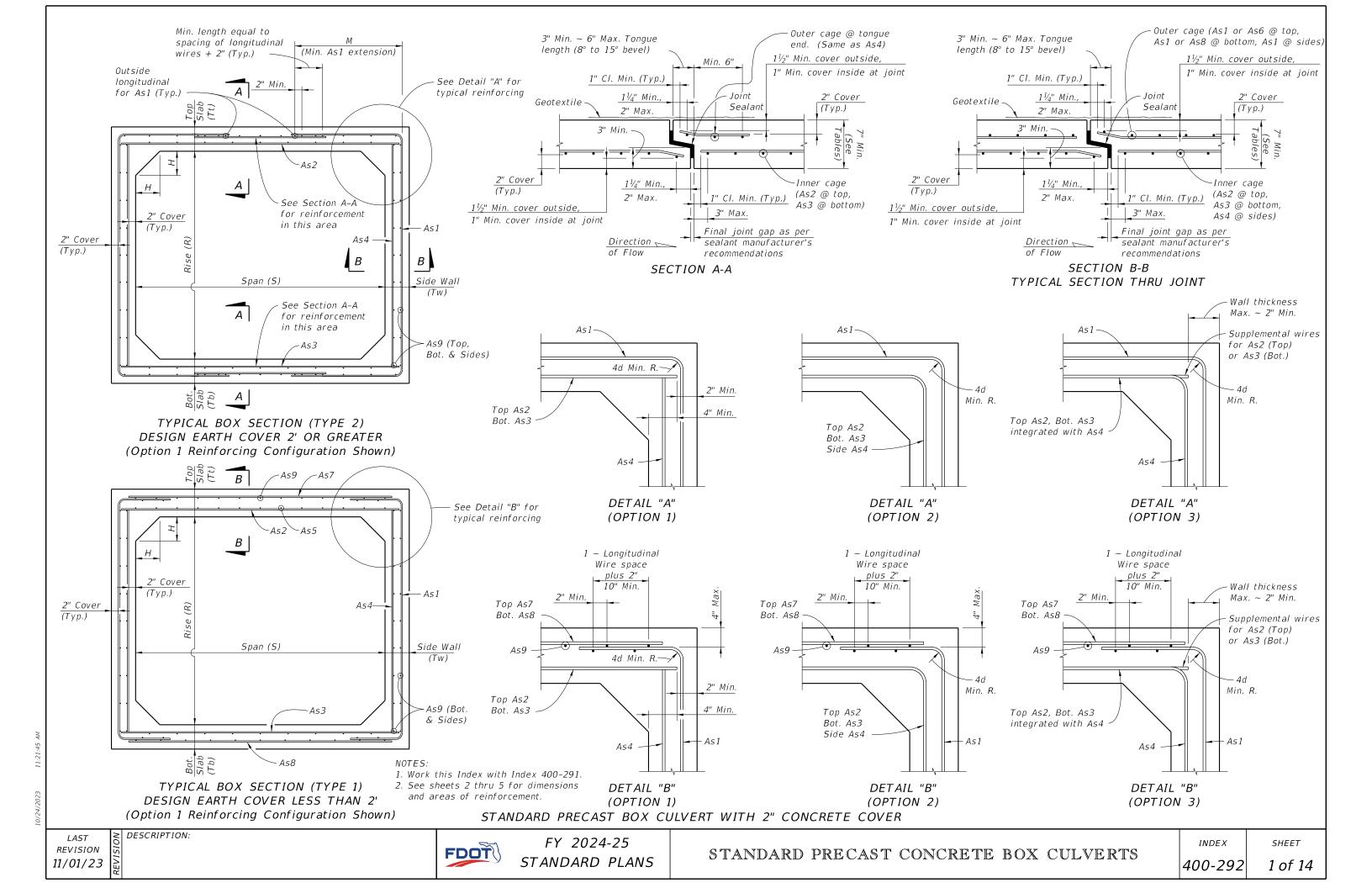




DIFFERENTIAL SETTLEMENT COUNTERMEASURES FOR PRECAST BOX CULVERTS =

DESCRIPTION:

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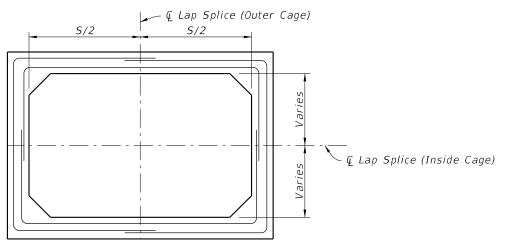


GENERAL NOTES:

- 1. These precast designs may be substituted for cast-in-place box culverts designed to AASHTO LRFD Bridge Design Specifications, 4th Edition. Designs are based on the design criteria shown in FDOT Structures Design Guidelines.
- 2. Loading: HL-93 & any fill heights between the minimum & maximum shown.
- 3. Only one design of precast box culvert is to be used for any installation.
- 4. Reinforcing steel must consist of smooth or deformed welded wire reinforcement (WWR) meeting the requirements of Specification Section 931. Longitudinal reinforcement may consist of reinforcing bars meeting the requirements of Specification Section 931. Minimum cover must be 2" for slightly or moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. The spacing of circumferential wires must not be less than 2" nor more than 4". The spacing of longitudinal wires or bars must not be more than 8".
- 5. As9 longitudinal wires must have a minimum cross-sectional area of 40% of the circumferential wires, but not less than a W2.5 or D4.0 for WWR, or #3 bars for deformed bars.
- 6. Welding of reinforcement must be limited to the locations shown in ASTM C1577 and in accordance with ANSI/AWS D1.4 "Structural Welding Code - Reinforcing Steel".
- 7. For alternate reinforcing configuration Options 2 and 3 shown in Detail "A" and "B" (Sheet 1), As1 may be extended to the middle of either slab and lap spliced with As7 and As8. As4 may be lap spliced at any location or connected to As2 or As3 at corners by welding.
- 8. Haunch dimensions may vary between the minimum and maximum dimensions shown in the Design Tables but only one haunch dimension must be used within the full length of the box culvert installation.

TABLE	1A	STANI	DARD	PRECA	ST BOX CU	LVERT	DES	SIGNS	(2" (COVE	R) - 3	' & ·	4' SP.	ANS
SPAN x RISE (S) (R)	SLAE TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		15		As1 EXT LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	A58	A59	(in.)
					0.33' - <2'	0.17	0.29	0.21	0.17	0.17	0.17	0.17		_
				4	2' - <3'	0.13	0.28	0.21	0.09	-	-	-		31
					3' - <5'	0.09	0.17	0.17	0.09	ı	-	-		31
					5' - 10'	0.09	0.17	0.17	0.09	ı	-	-		31
3' x 3'	7	7	7	to	15'	0.09	0.17	0.17	0.09	ı	-	-		31
					20'	0.12	0.17	0.17	0.09	ı	-	-		31
					25'	0.14	0.18	0.18	0.09	ı	-	-		31
				8	30'	0.17	0.21	0.22	0.09	-	-	-		31
					<i>35</i> ′	0.19	0.25	0.25	0.09	1	-	-	. 2	31
					0.33' - <2'	0.19	0.38	0.26	0.17	0.19	0.17	0.19	· ·	_
				4	2' - <3'	0.19	0.38	0.26	0.09	-	-	-	Note	38
				7	3' - <5'	0.14	0.20	0.22	0.09	-	-	-		38
4' x 3'	7	7	7	to	5' - 10'	0.11	0.17	0.17	0.09	ı	-	-	General	38
7 / 3	/	′	′		15'	0.15	0.17	0.18	0.09	ı	-	-	en	38
				8	20'	0.20	0.23	0.23	0.09	ı	-	-		38
					25'	0.24	0.28	0.29	0.09	-	-	-	See	38
					30'	0.29	0.34	0.35	0.09	ı	-	-		38
					0.33' - <2'	0.19	0.41	0.28	0.17	0.21	0.17	0.19		_
				4	2' - <3'	0.19	0.41	0.28	0.09	-	-	-		38
				7	3' - <5'	0.14	0.21	0.24	0.09	-	-	-		38
4' x 4'	7	7	7	to	5' - 10'	0.12	0.17	0.17	0.09	-	-	-		38
7 / 7	′	′	 ′		15'	0.16	0.19	0.20	0.09	-	-	-		38
				8	20'	0.21	0.25	0.25	0.09	1	-	-		38
					25'	0.26	0.31	0.32	0.09	1	-	-		38
					30'	0.31	0.37	0.38	0.09	-	-	-		38

- 9. Submittal of redesign calculations are not required for any increase to the slab and/or wall thickness when the minimum reinforcement areas shown in the Design Tables are provided.
- 10. For Design Earth Cover greater than 10 feet, the Contractor may interpolate the required areas of reinforcement and slab or wall thickness. Interpolated areas of reinforcement, slab or wall thickness must be approved by the Engineer.
- 11. Minimum length of precast box segments is 4 feet and maximum length is 16 feet.
- 12. See Index 400-291 for connections to wingwalls, headwalls and other general details.



SCHEMATIC OF LAP SPLICE LOCATIONS FOR OPTION 2 & 3 REINFORCING CONFIGURATIONS

TABL	.E 1B	- STA	ANDAR	RD PRE	CAST BOX	CULVE	ERT D	PESIG	NS (2	" COV	/ER) -	- 3'	& 4'	SPANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
,	(Tt)	(Tb)	(Tw)	(H)	ABOV E									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.20	0.26	0.32	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.16	0.25	0.31	0.10	1	-	-		31
					3' - <5'	0.10	0.20	0.20	0.10	1	-	_		31
					5' - 10'	0.10	0.20	0.20	0.10	ı	-	-		31
3' x 3'	8	8	8	to	15'	0.10	0.20	0.20	0.10	ı	-	-		31
					20'	0.10	0.20	0.20	0.10	ı	-	_		31
					25'	0.11	0.20	0.20	0.10	ı	-	-		31
				8	30'	0.13	0.20	0.20	0.10	-	-	_		31
					<i>35</i> ′	0.15	0.21	0.21	0.10	-	-	_	. 2	31
					0.33' - <2'	0.20	0.31	0.22	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.12	0.31	0.22	0.10	-	-	-	Note	38
				7	3' - <5'	0.12	0.20	0.20	0.10	-	-	_	-	38
4' x 3'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	General	38
7 / 3					15'	0.12	0.20	0.20	0.10	-	-	-	en	38
				8	20'	0.16	0.20	0.20	0.10	1	-	-		38
					25'	0.19	0.24	0.24	0.10	ı	-	-	See	38
					30'	0.22	0.28	0.29	0.10	1	-	-		38
					0.33' - <2'	0.20	0.33	0.24	0.20	0.20	0.20	0.20		_
				4	2' - <3'	0.17	0.33	0.24	0.10	-	-	_		38
				7	3' - <5'	0.12	0.20	0.20	0.10	1	-	-		38
4' x 4'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-		38
					15'	0.13	0.20	0.20	0.10	-	-	_		38
				8	20'	0.16	0.21	0.22	0.10	1	-	-		38
					25'	0.20	0.26	0.27	0.10	-	-	-		38
					30'	0.23	0.31	0.32	0.10	ı	-	-		38

NOTES: 1. See Sheet 1 for Reinforcing Details and dimension locations.

DESCRIPTION:

TABL	.E 2A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT L	DESIG	NS (2	?" CO	VER)	- 5'	& 6'	SPANS
SPAN x RISE	SLAB		L THIC		DESIGN			R	EINFOR			15		As1 EXT.
(S) (R)	TOP	BOT.	ı	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB		1	1	1		1		1	(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	As9	(111.)
					0.33' - <2'	0.31	0.48	0.42	0.17	0.21	0.23	0.31		-
				4	2' - <3'	0.31	0.48	0.42	0.09	-	-	-		45
					3' - <5'	0.20	0.27	0.27	0.09	-	-	-		36
5' x 3'	7	7	7	to	5' - 10'	0.17	0.19	0.21	0.09	-	-	_		36
					15'	0.24	0.25	0.25	0.09	-	-	-		35
				8	20'	0.32	0.33	0.33	0.09	-	-	-		35
					25'	0.39	0.41	0.42	0.09	-	-	-		35
					30'	0.47	0.50	0.50	0.09	-	-	-		35
					0.33' - <2'	0.30	0.51	0.45	0.17	0.23	0.21	0.30		-
				4	2' - <3'	0.30	0.51	0.45	0.09	-	-	-		45
	_	_	_		3' - <5'	0.18	0.30	0.29	0.09	-	-	-		45
5' x 4'	7	7	7	to	5' - 10'	0.17	0.21	0.23	0.09	-	-	-		36
					15'	0.24	0.27	0.28	0.09	-	-	-		35
				8	20'	0.31	0.36	0.37	0.09	-	-	-		35
					25'	0.39	0.45	0.46	0.09	-	-	-		35
					30'	0.46	0.55	0.56	0.09	-	-	-		35
					0.33' - <2'	0.30	0.53	0.48	0.17	0.24	0.21	0.30	-	-
				4	2' - <3'	0.29	0.53	0.48	0.09	_	_	-	-	45
	_	_	_		3' - <5'	0.19	0.31	0.31	0.09	-	-	-		45
5' x 5'	7	7	7	to	5' - 10'	0.19	0.22	0.25	0.09	-	-	-		45
					15'	0.26	0.29	0.31	0.09	-	-	-		36
				8	20'	0.34	0.39	0.40	0.09	-	-	-		35
					25'	0.41	0.49	0.50	0.09	-	-	-		35
					30'	0.49	0.59	0.61	0.09	-	-	-		35
_	7.5	7	7		0.33' - <2'	0.39	0.54	0.48	0.17	0.22	0.25	0.39	.5	-
				4	2' - <3'	0.39	0.58	0.49	0.09	-	_	-	General Note	43
	_	_	_		3' - <5'	0.28	0.36	0.36	0.09	-	-	-	2	39
6' x 3'	7	7	7	to	5' - 10'	0.25	0.26	0.28	0.09	-	-	-	ra	39
					15'	0.36	0.34	0.34	0.09	-	-	-	ene	38
_	_		_	12	20'	0.47	0.46	0.46	0.09	_	-	-	35	38
-	7	7.5	7		25'	0.59	0.57	0.55	0.09	-	-	-	See	38
	8	8	7		30'	0.60	0.64	0.64	0.09	-	-	-	- 0,	38
-	7.5	7	7		0.33' - <2'	0.37	0.58	0.52	0.17	0.24	0.23	0.37	-	-
				4	2' - <3'	0.37	0.61	0.53	0.09	-	-	-	-	43
C1 41	_	7			3' - <5'	0.26	0.39	0.39	0.09	-	-	-		39
6' x 4'	7	7	7	to	5' - 10'	0.24	0.28	0.31	0.09	-	-	-	-	39
					15'	0.35	0.37	0.38	0.09	-	_	-	-	38
_	7	7 -	7	12	20'	0.46	0.50	0.50	0.09	-	-	-		38
-	7 8	7.5	7		25' 30'	0.56	0.63	0.60	0.09	-	-	-		38 38
		<i>8 7</i>	7			0.58				0.25	- 0.22	0.26		-
-	7.5	/			0.33' - <2'	0.36	0.60	0.56	0.17	0.25	0.22	0.36		
				4	2' - <3'	0.36	0.64	0.56	0.09	-	-	-		43
61 51	_	7	_		3' - <5'	0.26	0.410	0.42	0.09	-	-	-	-	43
6' x 5'	7	7	7	to	5' - 10'	0.25	0.30	0.33	0.09	-	-	-	-	39 38
				1.0	15' 20'	0.34	0.40	0.41	0.09	-	-	-		38
-	7	7.5	7	12	25'	0.46	0.54	0.54	0.09	-	-	_	-	38
-	8	7.5 8	8		30'	0.56	0.67	0.65	0.09	-			-	38
	7.5	7	7								- 0.22	- 0.26	-	-
-	1.5		- 		0.33' - <2'	0.36	0.63	0.59	0.17	0.26	0.22	.036	-	
				4	2' - <3'	0.35	0.67	0.59	0.09	-	-	-		52
61 61	_	7	_		3' - <5'	0.27	0.43	0.44	0.09	-	-	-		52
6' x 6'	7	7	7	to	5' - 10'	0.27	0.32	0.35	0.09	-	-	-		43
I					15'	0.38	0.43	0.44	0.09	-	-	-		39
			I	12	20'	0.50	0.57	0.59	0.09	-	-	-	1	39
	7	7 -	7	1	25	0.00	070	0.70	0.00					20
	7 8	7.5 8	7 7		25' 30'	0.60 0.67	0.72 0.78	0.70 0.79	0.09	-	-	-	-	38 38

PAN x RISE (S) (R)	SLAB TOP (Tt)	BOT. (Tb)		KNESS HAUNCH (H)	<i>ABOVE</i>			R	EINFOR (s	RCEMEN q. in./F		15		As1 EX LENGT (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.26	0.39	0.36	0.20	0.20	0.20	0.26	,,,,,,	_
				4	2' - <3'	0.26	0.39	0.36	0.10	_	_	_		45
				7	3' - <5'	0.16	0.23	0.24	0.10	_	_	_		36
5' x 3'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	-	-	-		36
					15'	0.19	0.21	0.22	0.10	-	-	-		35
				8	20'	0.24	0.28	0.28	0.10	-	-	-		35
					25'	0.30	0.34	0.35	0.10	-	-	-		35
					30'	0.36	0.41	0.41	0.10	-	-	-		35
					0.33' - <2'	0.25	0.42	0.38	0.20	0.20	0.20	0.25		_
				4	2' - <3'	0.25	0.42	0.38	0.10	-	-	-		45
				,	3' - <5'	0.16	0.25	0.25	0.10	-	-	-		45
5' x 4'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	-	-	-		36
					15'	0.19	0.23	0.24	0.10	-	-	-		35
				8	20'	0.24	0.30	0.31	0.10	-	-	-		35
					25'	0.30	0.37	0.38	0.10	-	-	-		35
					30'	0.35	0.45	0.46	0.10	-	-	-		35
					0.33' - <2'	0.25	0.44	0.41	0.20	0.20	0.20	0.25		_
				4	2' - <3'	0.25	0.44	0.41	0.10	-	-	-		45
					3' - <5'	0.16	0.26	0.27	0.10	-	-	-		45
5' x 5'	8	8	8	to	5' - 10'	0.15	0.20	0.22	0.10	-	-	-		45
					15'	0.20	0.25	0.26	0.10	-	-	-		36
				8	20'	0.26	0.32	0.33	0.10	-	-	_		35
					25'	0.32	0.40	0.41	0.10	-	-	-		35
					30'	0.37	0.48	0.49	0.10	-	-	-		35
					0.33' - <2'	0.32	0.47	0.41	0.20	0.20	0.25	0.32	5	_
				4	2' - <3'	0.32	0.47	0.41	0.10	-	-	-	Note	43
					3' - <5'	0.23	0.30	0.31	0.10	-	-	-	ž	39
6' x 3'	8	8	8	to	5' - 10'	0.19	0.22	0.24	0.10	-	-	-	General	39
					15'	0.28	0.29	0.29	0.10	-	-	-	ne	38
				12	20'	0.36	0.38	0.38	0.10	-	-	-		38
					25 ¹	0.45	0.47	0.47	0.10	-	-	-	See	38
					30'	0.54	0.57	0.57	0.10	-	-	-	S	38
					0.33' - <2'	0.31	0.50	0.44	0.20	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.50	0.44	0.10	-	-	-		43
					3' - <5'	0.23	0.32	0.34	0.10	-	-	-		39
6' x 4'	8	8	8	to	5' - 10'	0.19	0.24	0.26	0.10	-	-	-		39
					15'	0.27	0.31	0.32	0.10	-	-	-		38
				12	20'	0.35	0.41	0.41	0.10	-	-	-		38
					25'	0.43	0.51	0.51	0.10	-	-	-		38
					30'	0.52	0.62	0.62	0.10	-	-	- 0.20		38
					0.33' - <2'	0.30	0.52	0.47	0.20	0.22	0.22	0.30		- 42
				4	2' - <3'	0.30	0.52	0.47	0.10	-	-	_		43
GI v EI	0	,	,		3' - <5'	0.22	0.34	0.36	0.10	-	-	_		43
6' x 5'	8	8	8	to	5' - 10' 15'	0.20	0.26	0.28	0.10		-			39 38
				1.3	20'	0.27	0.33	0.34	0.10	-	-	-		38
				12	20° 25'	0.36 0.44	0.44	0.45	0.10	-	-			38
					30'	0.44	0.55	0.55	0.10		-	-		38
					0.33' - <2'		0.54	0.50						-
					0.33 - <2 2' - <3'	0.30	0.54	0.50	0.20	0.22	0.22	0.30		52
				4	2 - <3 3' - <5'	0.30	0.36	0.30	0.10	_		-		52
6' x 6'	8	8	8	+-	5' - 10'	0.23	0.36	0.30	0.10	_	-	_		43
0 1 0				to	15'	0.21	0.27	0.37	0.10	_	_	_		39
				12	20'	0.29	0.33	0.37	0.10	_	-	_		39
				12	25'	0.38	0.47	0.48	0.10	_	_	_		38
					30'	0.47	0.70	0.71	0.10	_	_	_		38

LAST REVISION 07/01/13

≥ DESCRIPTION:

FDOT

T	ABLE	3 - 5	STANI	DARD P	PRECAST BO	X CU	LVERT	DES	GIGNS	(2" (COVER	R) - 7	SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	ВОТ.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.37	0.58	0.49	0.20	0.22	0.29	0.37		-
				4	2' - <3'	0.37	0.58	0.49	0.10	-	-	-		43
				to	3' - <5'	0.30	0.40	0.42	0.10	-	_	-		43
7' x 4'	8	8	8	10	5' - 10'	0.26	0.30	0.33	0.10	_	-	-		43
				12	15'	0.37	0.40	0.40	0.10	-	-	-		41
				12	20'	0.49	0.53	0.53	0.10	-	-	-		41
	8	8	8	7 to	25'	0.60	0.67	0.66	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.68	0.79	0.78	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.60	0.53	0.20	0.23	0.28	0.36		-
				4	2' - <3'	0.36	0.60	0.53	0.10	-	_	-		47
					3' - <5'	0.30	0.42	0.45	0.10	-	_	-		43
7' x 5'	8	8	8	to	5' - 10'	0.26	0.32	0.35	0.10	-	-	-		43
				12	15'	0.37	0.43	0.44	0.10	-	-	-	5	41
				12	20'	0.48	0.57	0.57	0.10	-	-	-	te	41
	8	8	8	7 to	25'	0.60	0.72	0.72	0.10	_	_	-	No	41
	8.5	8.5	8	12	30'	0.67	0.84	0.84	0.10	-	-	-	le.	41
				4	0.33' - <2'	0.36	0.63	0.56	0.20	0.24	0.27	0.36	General Note	-
				4	2' - <3'	0.36	0.63	0.56	0.10	-	-	-	Ge.	59
					3' - <5'	0.29	0.44	0.47	0.10	-	-	-	See	47
7' x 6'	8	8	8	to	5' - 10'	0.27	0.34	0.37	0.10	-	-	-	Se	43
				12	15'	0.38	0.46	0.46	0.10	-	-	-		41
				12	20'	0.49	0.60	0.61	0.10	-	-	-		41
	8	8	8	7 to	25'	0.61	0.76	0.76	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.69	0.89	0.89	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.65	0.58	0.20	0.25	0.27	0.36		-
				4	2' - <3'	0.36	0.65	0.58	0.10	-	_	-		59
					3' - <5'	0.30	0.46	0.50	0.10	-	-	-		59
7' x 7'	8	8	8	to	5' - 10'	0.30	0.35	0.50	0.10	-	-	-		47
				12	15'	0.41	0.48	0.50	0.10	-	-	-		43
				12	20'	0.53	0.64	0.65	0.10	-	-	-		43
	8	8	8	7 to	25'	0.65	0.80	0.81	0.10	-	-	-		43
	8.5	9	8	12	30'	0.72	0.92	0.91	0.10	-	-	-		41

7	ABLE	4 - 5	STANI	DARD P	RECAST BO	X CU	LV ER 7	DES	IGNS	(2" C	COVER	R) - 8	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOF	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	I	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M)
(ГГ.)	(in.)	(in.)	(in.)	(in.)	TUP SLAD	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
	9	8.5	8	4	0.33' - <2'	0.40	0.60	0.52	0.20	0.22	0.28	0.39		-
				, '	2' - <3'	0.45	0.66	0.54	0.10	-	-	-		50
				to	3' - <5'	0.39	0.48	0.50	0.10	-	-	-		50
8' x 4'	8	8	8		5' - 10'	0.34	0.38	0.40	0.10	-	-	-		45
				12	15'	0.49	0.51	0.50	0.10	-	-	-		41
					20'	0.65	0.68	0.66	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.76	0.83	0.80	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.79	0.94	0.92	0.10	-	-	-		41
	9	8.5	8	4	0.33' - <2'	0.38	0.65	0.59	0.20	0.22	0.30	0.37		-
				, '	2' - <3'	0.43	0.69	0.58	0.10	-	-	-		50
				to	3' - <5'	0.37	0.51	0.53	0.10	-	-	-		45
8' x 5'	8	8	8		5' - 10'	0.33	0.41	0.42	0.10	-	-	-		45
				12	15'	0.48	0.54	0.53	0.10	-	-	-		41
					20'	0.63	0.73	0.70	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.74	0.88	0.86	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.77	1.00	0.98	0.10	-	-	-		41
	9	9	8	4	0.33' - <2'	0.32	0.65	0.58	0.20	0.23	0.25	0.31	5	-
				'	2' - <3'	0.42	0.71	0.61	0.10	-	-	-	Note	50
				to	3' - <5'	0.37	0.54	0.56	0.10	-	-	-		50
8' x 6'	8	8	8		5' - 10'	0.34	0.43	0.45	0.10	-	-	-	General	45
				12	15'	0.49	0.57	0.57	0.10	-	-	-	ne	41
					20'	0.64	0.77	0.76	0.10	-	-	-	95	41
	8.5	8.5	8	8 to	25'	0.74	0.94	0.92	0.10	-	-	-	See	41
	9.5	9.5	8	12	30'	0.78	1.05	1.04	0.10	-	-	-	S	41
	9	9	8	4	0.33' - <2'	0.31	0.67	0.60	0.20	0.24	0.24	0.31		-
				'	2' - <3'	0.42	0.74	0.64	0.10	-	-	-		55
				to	3' - <5'	0.37	0.56	0.59	0.10	-	-	-		55
8' x 7'	8	8	8		5' - 10'	0.36	0.45	0.47	0.10	-	-	-		50
				12	15'	0.51	0.61	0.61	0.10	-	-	-		45
					20'	0.66	0.81	0.80	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.78	0.98	0.97	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.84	1.10	1.09	0.10	-	-	-		41
	9	9	8	4	0.33' - <2'	0.32	0.68	0.62	0.20	0.24	0.25	0.32		-
				7	2' - <3'	0.43	0.76	0.67	0.14	-	-	-		65
				to	3' - <5'	0.38	0.58	0.61	0.14	-	-	-		65
8' x 8'	8	8	8		5' - 10'	0.39	0.46	0.50	0.13	-	-	-		55
				12	15'	0.55	0.64	0.65	0.10	-	-	-		45
				12	20'	0.71	0.86	0.85	0.10	-	-	-		45
	8.5	8.5	8	8 to	25'	0.84	1.03	1.02	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.93	1.15	1.15	0.10	-	-	-		41

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

≥ DESCRIPTION: REVISION 07/01/13



TABL	.E 5 -	- STA	NDAR	D PREC	CAST BOX (CULVE	RT DI	ESIGN	IS (2"	COV	ER) -	9' SP	ANS	
SPAN x RISE			LTHIC		DESIGN EARTH COVER			R		RCEMEN		15		AS1 EXT.
(S) (R)	T0P (T+)	BOT.	l	HAUNCH	ABOVE				(5	q. in./F	L.)			LENGTH (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	1 - 7	4-2	4-2	1 - 1	1	4 - 7	4 - 0	4-0	(in.)
. ,	9.5	9.5	9	(,,,,,	0.33' - <2'	As1	<i>As2</i> 0.62	As3 0.53	As4 0.22	As5 0.23	<i>As7</i> 0.34	As8 0.38	As9	
	9.5	9.5	9	4	2' - <3'	0.41	0.65	0.54	0.22	0.23	0.54	-		54
					3' - <5'	0.39	0.53	0.51	0.11	_		_		49
9' x 5'	9	9	9	to	5' - 10'	0.35	0.42	0.44	0.11	_	_	_		49
					15'	0.50	0.56	0.55	0.11	_	_	_		44
				12	20'	0.65	0.75	0.73	0.11	-	_	-		44
	9.5	9.5	9	8 to	25'	0.77	0.92	0.90	0.11	_	_	-		44
	10.5	11	9	12	30'	0.81	1.05	1.02	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.38	0.64	0.56	0.23	0.23	0.33	0.37		_
				4	2' - <3'	0.43	0.67	0.57	0.11	-	-	-		54
				to	3' - <5'	0.37	0.55	0.54	0.11	-	-	-		49
9' x 6'	9	9	9	"	5' - 10'	0.35	0.45	0.47	0.11	-	-	-		49
				12	15'	0.49	0.60	0.59	0.11	-	-	-		44
				12	20'	0.65	0.80	0.78	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.76	0.98	0.95	0.11	-	-	-		44
	10.5	11	9	12	30'	0.80	1.10	1.08	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.37	0.67	0.59	0.22	0.23	0.32	0.37	5	-
				,	2' - <3'	0.42	0.69	0.60	0.11	-	-	-	Note	59
				to	3' - <5'	0.37	0.58	0.56	0.11	-	-	-		54
9' x 7'	9	9	9		5' - 10'	0.36	0.47	0.49	0.11	-	-	-	General	49
				12	15'	0.50	0.63	0.63	0.11	-	-	-	eue	44
					20'	0.66	0.84	0.80	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.77	1.02	1.00	0.11	-	-	-	See	44
	10.5	11	9	12	30'	0.81	1.15	1.13	0.11	-	-	-	8	44
	9.5	9.5	9	4	0.33' - <2'	0.37	0.68	0.61	0.22	0.23	0.31	0.37		-
					2' - <3'	0.42	0.71	0.62	0.11	-	-	-		59
				to	3' - <5'	0.37	0.60	0.59	0.11	-	-	-		59
9' x 8'	9	9	9		5' - 10'	0.38	0.49	0.51	0.11	-	-	-		54
				12	15'	0.53	0.66	0.66	0.11	=	=	-		44
	9.5	9.5	9	8 to	20' 25'	0.68	0.88 1.07	0.87 1.05	0.11	-	-	-		44
	10.5	9.5	9	12	30'	0.81	1.20	1.18	0.11	_		_		44
	9.5	9.5	9	12	0.33' - <2'	0.88	0.70	0.63	0.11	0.23	0.32	0.38		-
	9.5	9.3	9	4	0.33 - <2 2' - <3'	0.38	0.70	0.65	0.22	0.23	0.32	0.38		72
					2 - < 3 3' - < 5'	0.43	0.73	0.63	0.15	_	_	_		72
9' x 9'	9	9	9	to	5' - 10'	0.41	0.50	0.53	0.13	_	_	_		59
					15'	0.41	0.69	0.70	0.14	_	_	_		49
				12	20'	0.73	0.92	0.91	0.12	_		_		49
	9.5	10	9	8 to	25'	0.83	1.11	1.09	0.11	_	_	_		44
	10.5	11	9	12	30'	0.93	1.25	1.23	0.11	_	_	_		44
	30.0					1 0.00				I				1

TABI					AST BOX C	ULVE	RT DE						PANS	_
SPAN x RISE		/ WAL	L THIC	KNESS	DESIGN			R			T AREA	15		As1 EXT
(S) (R)	TOP	BOT.	SIDE	HAUNCH					(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.46	0.62	0.52	0.24	0.24	0.41	0.45	7,100	_
				4	2' - <3'	0.46	0.62	0.52	0.12	-	-	-		58
					3' - <5'	0.42	0.54	0.50	0.12	_	_	_		53
101 v El	10	10	10	to	5' - 10'	0.42	0.46	0.49	0.12					52
10' x 5'	10	10	10							-	-	_		
				12	15'	0.52	0.59	0.58	0.12	-	_	-		47
					20'	0.69	0.78	0.76	0.12	-	_	-		47
	10.5	10.5	10	8 to	25'	0.81	0.97	0.93	0.12	-	-	-		47
	11.5	12	10	12	30'	0.87	1.11	1.11	0.12	-	-	-		47
				4	0.33' - <2'	0.44	0.64	0.54	0.24	0.24	0.39	0.44		_
				7	2' - <3'	0.44	0.64	0.54	0.12	-	_	-		58
				t o	3' - <5'	0.39	0.57	0.52	0.12	_	-	_		52
10' x 6'	10	10	10	to	5' - 10'	0.37	0.48	0.52	0.12	-	-	-		52
				1.3	15'	0.51	0.62	0.61	0.12	-	-	-		47
				12	20'	0.67	0.83	0.80	0.12	-	-	-		47
	10.5	10.5	10	8 to	25'	0.79	1.02	0.99	0.12	_	_	_		47
	11.5	12	10	12	30'	0.85	1.17	1.14	0.12	_	_	_		47
	11.5	12	10	12	0.33' - <2'	0.43	0.66	0.57	0.24	0.24	0.38	0.43		-
				4	2' - <3'		0.66	0.57	0.24	-				
						0.43					-	-		58
				to	3' - <5'	0.38	0.59	0.55	0.12	-	-	-		58
10' x 7'	10	10	10		5' - 10'	0.37	0.50	0.54	0.12	-	_	-		52
				12	15'	0.52	0.66	0.65	0.12	-	-	-	5	47
					20'	0.67	0.87	0.85	0.12	-	-	-	Note	47
	10.5	10.5	10	8 to	25'	0.79	1.07	1.04	0.12	-	-	-	Nc	47
	11.5	12	10	12	30'	0.84	1.22	1.19	0.12	-	_	_	General	47
				1	0.33' - <2'	0.43	0.68	0.60	0.24	0.24	0.38	0.43	ıeı	_
				4	2' - <3'	0.43	0.68	0.60	0.12	_	_	-	3e1	64
					3' - <5'	0.38	0.62	0.57	0.12	_	_	_		58
10' x 8'	10	10	10	to	5' - 10'	0.38	0.52	0.57	0.12	_	_	_	See	52
10 % 0	'		10		15'	0.53	0.69	0.68	0.12	_	_	_		47
				12	20'	0.68	0.03	0.89	0.12	_	_	_		47
	10 5	10.5	10	8 to	25'	0.81	1.12	1.09	0.12	-	-	_		47
	10.5									_	_	_		
	11.5	12	10	12	30'	0.86	1.27	1.25	0.12	-	-	-		47
				4	0.33' - <2'	0.43	0.70	0.62	0.24	0.24	0.38	0.43		-
					2' - <3'	0.43	0.70	0.62	0.12	-	-	-		70
				to	3' - <5'	0.39	0.64	0.60	0.12	-	-	-		64
10' x 9'	10	10	10		5' - 10'	0.40	0.54	0.59	0.12	-	-	-		58
				12	15'	0.56	0.72	0.72	0.12	-	-	-		52
				12	20'	0.71	0.95	0.94	0.12	-	-	-		47
	10.5	11	10	8 to	25'	0.82	1.15	1.13	0.12	-	-	-		47
	11.5	12	10	12	30'	0.90	1.32	1.30	0.12	_	-	-		47
	1		-		0.33' - <2'	0.44	0.71	0.64	0.24	0.24	0.38	0.44		_
				4	2' - <3'	0.44	0.71	0.64	0.17	-	-	-		79
					3' - <5'	0.40	0.65	0.62	0.17					70
101 . 101	1.0	10	10	to						_	_	_		
10' x 10'	10	10	10		5' - 10'	0.44	0.56	0.61	0.15	-	-	-		64
				12	15'	0.60	0.75	0.76	0.12	-	-	-		52
					20'	0.76	0.99	0.99	0.12	-	-	-		52
	10.5	11	10	8 to	25'	0.86	1.20	1.18	0.12	-	-	-		47
	11.5	12	10	12	30'	0.97	1.36	1.35	0.13	-	-	-		47

- See Sheet 1 for Reinforcing Details and dimension locations.
 See Sheet 2 for General Notes.
 See Sheet 14 for WWR Bending Diagram.

≥ DESCRIPTION: LAST REVISION 07/01/13



SPAN x RISE		- 31 A R / WAL			CAST BOX (ULVE	תו ט				T AREA		FANS	As1 EXT
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER			K		q. in./F		13		LENGTH
(3)	(Tt)	(Tb)	(Tw)	(H)	ABOVE				(3	9. ,,,,,	,			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.52	0.57	0.45	0.29	0.29	0.47	0.49		_
				4	2' - <3'	0.52	0.57	0.45	0.15	_	_	_		73
					3' - <5'	0.50	0.54	0.45	0.15	_	_	_		66
12' x 4'	12	12	12	to	5' - 10'	0.50	0.52	0.52	0.15	-	_	-		66
				12	15'	0.63	0.61	0.59	0.15	-	-	-		59
				12	20'	0.82	0.81	0.77	0.15	-	-	-		59
	12.5	12.5	12	8 to	25'	0.99	0.99	0.95	0.15	-	-	-		59
	14	14	12	12	30'	1.03	1.15	1.11	0.15	-	-	-		59
				4	0.33' - <2'	0.47	0.62	0.51	0.29	0.29	0.42	0.46		_
				4	2' - <3'	0.47	0.62	0.51	0.15	-	-	-		66
				to	3' - <5'	0.45	0.60	0.51	0.15	-	-	-		59
12' x 6'	12	12	12	10	5' - 10'	0.47	0.59	0.59	0.15	-	-	-		59
				12	15'	0.57	0.68	0.66	0.15	_	_	-		53
				12	20'	0.74	0.90	0.86	0.15	-	-	-		53
	12.5	12.5	12	8 to	25'	0.88	1.11	1.06	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.27	1.24	0.15	-	-	-		53
				4	0.33' - <2'	0.44	0.67	0.56	0.29	0.29	0.40	0.44	5	_
				4	2' - <3'	0.44	0.67	0.56	0.15	-	-	-	te	66
				to	3' - <5'	0.41	0.64	0.56	0.15	-	-	-	Note	59
12' x 8'	12	12	12	10	5' - 10'	0.45	0.63	0.64	0.15	-	-	-		59
				12	15'	0.56	0.75	0.73	0.15	-	-	-	ner	53
				12	20'	0.72	0.98	0.95	0.15	-	-	-	General	53
	12.5	13	12	8 to	25'	0.85	1.20	1.16	0.15	-	-	-	See	53
	14	14.5	12	12	30'	0.89	1.38	1.35	0.15	-	-	-	Se	53
				4	0.33' - <2'	0.44	0.71	0.60	0.29	0.29	0.39	0.44		_
				7	2' - <3'	0.44	0.71	0.60	0.15	-	-	-		73
				to	3' - <5'	0.42	0.68	0.60	0.15	-	-	-		66
12' x 10'	12	12	12		5' - 10'	0.47	0.67	0.69	0.15	-	-	-		59
				12	15'	0.59	0.81	0.81	0.15	=	=	=		53
					20'	0.75	1.06	1.04	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.87	1.30	1.26	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.47	1.45	0.15	-	-	-		53
				4	0.33' - <2'	0.46	0.74	0.64	0.29	0.29	0.40	0.46		_
				'	2' - <3'	0.46	0.74	0.64	0.20	-	-	-		93
				to	3' - <5'	0.42	0.72	0.64	0.20	-	-	-		80
12' x 12'	12	12	12		5' - 10'	0.54	0.71	0.74	0.18	-	-	-		73
				12	15'	0.66	0.87	0.89	0.15	-	-	-		59
					20'	0.83	1.14	1.13	0.15	-	-	-		59
	12.5	13	12	8 to	25'	0.96	1.39	1.37	0.15	-	-	-		53
	14	14.5	12.5	12	30'	1.05	1.56	1.56	0.15	-	_	_		53

NOTES:

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

DESCRIPTION:



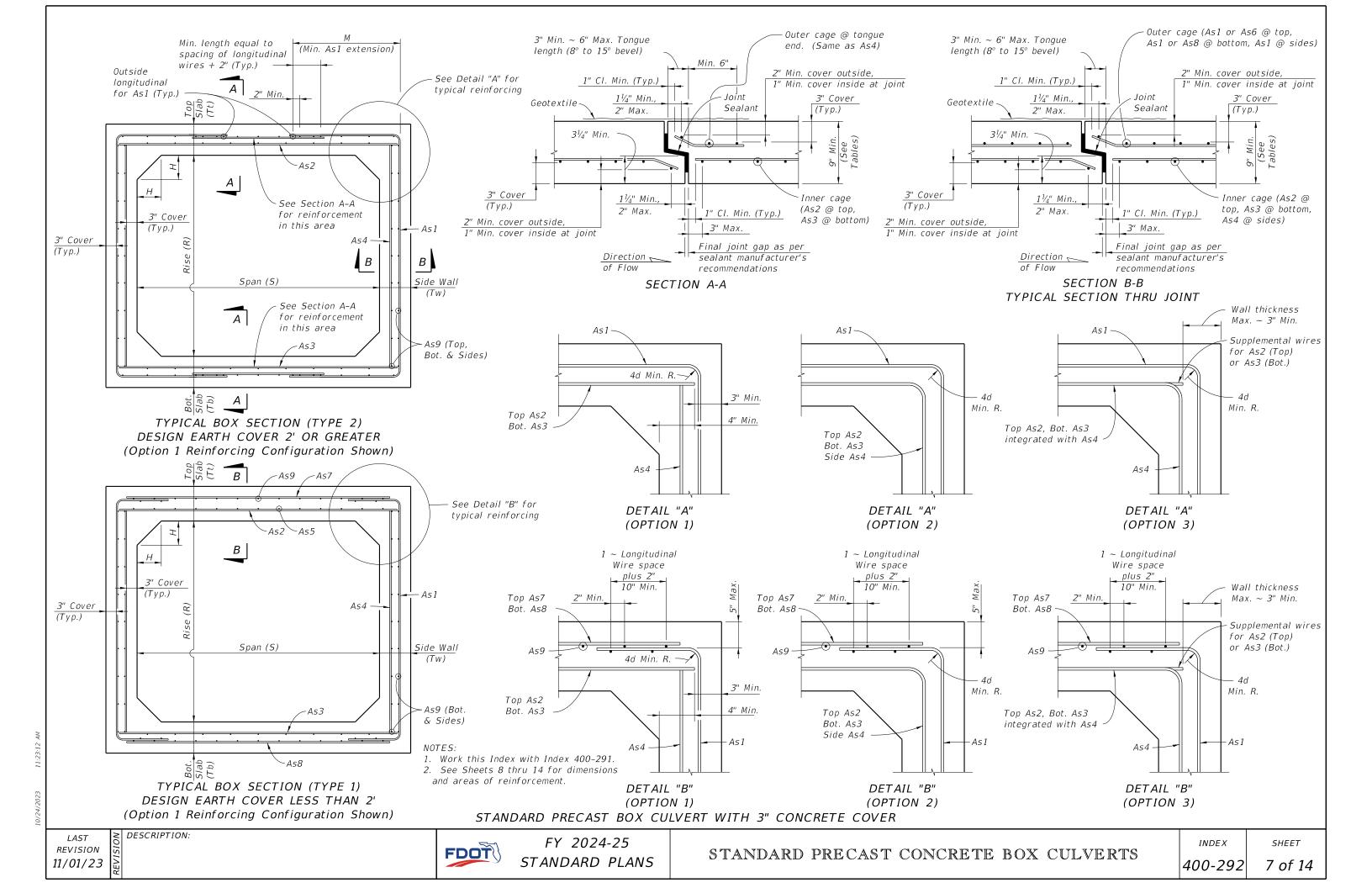


TABLE 9A - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 3' & 4' SPANS SPAN x RISE SLAB / WALL THICKNESS DESIGN REINFORCEMENT AREAS As1 EXT.														
SPAN x RISE (S) (R)	SLAE TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R		CEMEN q. in./F		15		As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.22	0.24	0.22	0.22	0.22	0.22	0.22		-
					2' - <3'	0.11	0.23	0.22	0.11	-	-	-		31
				4	3' - <5'	0.11	0.22	0.22	0.11	-	-	-		31
					5' - 10'	0.11	0.22	0.22	0.11	-	-	-		31
3' x 3'	9	9	9	to	15'	0.11	0.22	0.22	0.11	-	-	-		31
					20'	0.13	0.22	0.22	0.11	-	-	-		31
				8	25'	0.16	0.22	0.22	0.11	-	-	-		31
					30'	0.19	0.24	0.25	0.11	-	-	-		31
					<i>35</i> ′	0.22	0.28	0.29	0.11	-	-	-		31
					0.33' - <2'	0.22	0.32	0.24	0.22	0.22	0.22	0.22	5	-
				4	2' - <3'	0.17	0.31	0.24	0.11	-	-	-	Note	38
					3' - <5'	0.13	0.22	0.22	0.11	-	_	-		38
4' x 3'	9	9	9	to	5' - 10'	0.13	0.22	0.22	0.11	-	-	-	General	38
					15'	0.17	0.22	0.22	0.11	-	-	-	neı	38
				8	20'	0.23	0.26	0.27	0.11	-	-	-	Ge	38
					25'	0.28	0.32	0.34	0.11	-	-	-	ье	38
					30'	0.33	0.39	0.40	0.11	-	-	-	Š	38
					0.33' - <2'	0.22	0.34	0.26	022	0.22	0.22	0.22		_
				4	2' - <3'	0.17	0.33	0.26	0.11	-	-	-		38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 4'	9	9	9	to	5' - 10'	0.14	0.22	0.22	0.11	-	-	-		38
					15'	0.19	0.22	0.23	0.11	-	-	-		38
				8	20'	0.24	0.28	0.30	0.11	-	-	-		38
					25'	0.29	0.36	0.37	0.11	-	-	-		38
					30'	0.34	0.43	0.45	0.11	_	_	_		38

TABLE	9B -	STAN	DARD	PRECA	AST BOX CL	JLVER	T DE.	SIGNS	5 (3"	COVE	R) - 3	3' & 4	" SPA	ANS
SPAN x RISE (S) (R)	SLAB TOP	B / WAL		KNESS HAUNCH	DESIGN EARTH COVER			R		CEMEN q. in./F	T AREA	15		As1 EXT. LENGTH
(1.7)	(Tt)	(Tb)	(Tw)	(H)	AB0VE				, -	7	/			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.24	0.24	0.24	0.24	0.24	0.24		-
					2' - <3'	0.12	0.24	0.24	0.24	-	-	-		31
				4	3' - <5'	0.12	0.24	0.24	0.24	-	-	-		31
					5' - 10'	0.12	0.24	0.24	0.24	-	-	-		31
3' x 3'	10	10	10	to	15'	0.12	0.24	0.24	0.24	-	-	-		31
					20'	0.12	0.24	0.24	0.24	-	-	-		31
				8	25'	0.13	0.24	0.24	0.24	_	-	-		31
					30'	0.15	0.24	0.24	0.12	-	-	-		31
					35'	0.18	0.24	0.24	0.12	1	-	-		31
					0.33' - <2'	0.24	0.26	0.24	0.24	0.24	0.24	0.24	5	-
				4	2' - <3'	0.14	0.26	0.24	0.12	ı	-	-	Note	38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-		38
4' x 3'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-	eneral	38
					15'	0.14	0.24	0.24	0.12	-	-	-	neı	38
				8	20'	0.18	0.24	0.24	0.12	ı	-	-	<i>6</i> e	38
					25'	0.22	0.26	0.27	0.12	ı	-	-	өө	38
					30'	0.26	0.31	0.32	0.12	ı	-	-	Sı	38
					0.33' - <2'	0.24	0.28	0.24	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.14	0.28	0.24	0.12	-	-	-		38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-		38
4' x 4'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-		38
					15'	0.15	0.24	0.24	0.12	-	-	-		38
				8	20'	0.19	0.24	0.24	0.12	-	-	-		38
					25'	0.23	0.28	0.30	0.12	-	-	-		38
					30'	0.27	0.34	0.35	0.12	_	-	_		38

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: LAST REVISION 07/01/13



TABLE	10A -	STAN	IDARD	PREC	AST BOX C	JLVEF	RT DE	SIGN.	5 (3"	COVE	ER) -	5' & 6	5' SP.	ANS
SPAN x RISE	SLAE	/ WAL			DESIGN			R	EINFOR			15		As1 EXT.
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB				1		ı			(M) (in.)
(1 (.)	(111.)	(111.)	(111.)	(111.)		As 1	As2	A53	As4	As5	As7	A58	As9	
					0.33' - <2'	0.27	0.39	0.37	0.22	0.22	0.22	0.27		- 45
				4	2' - <3' 3' - <5'	0.26	0.39	0.37	0.11	_	-	_		45
5' x 3'	9	9	9	,	5' - 10'	0.19	0.24	0.25	0.11	-	-	-		36 36
<i>J</i>	9	9	9	to	15'	0.28	0.22	0.22	0.11	_	_	_		35
				8	20'	0.20	0.38	0.39	0.11	_	_	_		35
				0	25'	0.45	0.48	0.49	0.11	_	_	_		35
					30'	0.54	0.58	0.59	0.11	_	_	_		35
					0.33' - <2'	0.26	0.42	0.39	0.22	0.22	0.22	0.26		_
				4	2' - <3'	0.26	0.42	0.39	0.11	-	-	-		45
				,	3' - <5'	0.19	0.26	0.27	0.11	-	-	-		45
5' x 4'	9	9	9	to	5' - 10'	0.20	0.22	0.23	0.11	-	_	-		36
					15'	0.27	0.31	0.33	0.11	-	-	-		35
				8	20'	0.36	0.42	0.43	0.11	-	-	-		35
					25'	0.44	0.52	0.54	0.11	-	-	-		35
					30'	0.53	0.63	0.65	0.11	-	-	-		35
					0.33' - <2'	0.27	0.44	0.42	0.22	0.22	0.22	0.27		-
				4	2' - <3'	0.27	0.44	0.42	0.11	-	-	-		45
					3' - <5'	0.20	0.27	0.28	0.11	-	-	-		45
5' x 5'	9	9	9	to	5' - 10'	0.22	0.23	0.26	0.11	-	_	-		45
					15'	0.30	0.34	0.36	0.11	-	_	-		36
				8	20'	0.38	0.45	0.47	0.11	-	-	-		35
					25'	0.47	0.56	0.59	0.11	-	-	-		35
					30'	0.55	0.68	0.71	0.11	-	-	-		35
					0.33' - <2'	0.34	0.47	0.42	0.22	0.22	0.25	0.34	5	-
				4	2' - <3'	0.34	0.47	0.42	0.11	-	-	-	ote	43
					3' - <5'	0.27	0.31	0.32	0.11	-	_	-	General Note	39
6' x 3'	9	9	9	to	5' - 10'	0.29	0.26	0.28	0.11	-	-	-	ral	39
					15'	0.42	0.39	0.40	0.11	-	-	-	ene	38
				12	20'	0.55	0.52	0.53	0.11	_	-	-		38
					25'	0.68	0.66	0.67	0.11	-	-	-	See	38
					30'	0.82	0.81	0.82	0.11	-	-	-	0,	38
					0.33' - <2'	0.33	0.50	0.46	0.22	0.22	0.23	0.33		- 42
				4	2' - <3'	0.33	0.50	0.46	0.11	-	-	-		43
61 11				l .	3' - <5'	0.27	0.33	0.35	0.11	-	_	-		39
6' x 4'	9	9	9	to	5' - 10' 15'	0.28 0.40	0.29	0.31	0.11	-	-	-		39 38
				1 2	20'	0.40	0.43	0.45	0.11	-	-	-		38
				12	25'	0.52	0.37	0.74	0.11	_	_	_		38
					30'	0.78	0.73	0.90	0.11	_	_	_		38
					0.33' - <2'	0.33	0.52	0.49	0.22	0.22	0.23	0.33		-
				4	2' - <3'	0.33	0.52	0.49	0.22	-	-	-		43
				"	3' - <5'	0.27	0.35	0.43	0.11	_	_	_		43
6' x 5'	9	9	9	to	5' - 10'	0.29	0.31	0.34	0.11	_	_	-		39
				"	15'	0.41	0.46	0.49	0.11	_	-	_		38
				12	20'	0.53	0.62	0.64	0.11	_	_	-		38
				'-	25'	0.66	0.78	0.80	0.11	_	_	-		38
					30'	0.78	0.95	0.97	0.11	-	-	-		38
					0.33' - <2'	0.34	0.55	0.51	0.22	0.22	0.24	0.34		_
				4	2' - <3'	0.34	0.54	0.51	0.11	-	-	-		52
				,	3' - <5'	0.29	0.37	0.39	0.11	-	-	-		52
6' x 6'	9	9	9	to	5' - 10'	0.32	0.34	0.37	0.11	-	_	-		43
					15'	0.44	0.50	0.53	0.11	-	_	-		39
				12	20'	0.57	0.66	0.70	0.11	-	-	-		39
					25'	0.70	0.84	0.87	0.11	-	_	-		38
					30'	0.83	1.02	1.05	0.11	-	-	-		38

I ABLE	10B -	STAN	IDARE	PREC	AST BOX CU	JLVEF	RT DE	SIGN.	5 (3"	COVE	R) -	5' & 6	5' SP.	ANS
SPAN x RISE			L THIC		DESIGN			R		RCEMEN		45		As1 EX
(S) (R)	TOP	BOT.	l .	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB						I			(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	A58	A59	(111.)
					0.33' - <2'	0.24	0.33	0.32	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.22	0.33	0.32	0.12	-	-	-		45
EL 31	10	10	1.0		3' - <5'	0.16	0.24	0.24	0.12	-	-	-		36
5' x 3'	10	10	10	to	5' - 10' 15'	0.16	0.24	0.24	0.12	_	-	_		36 35
				1.2	20'	0.23	0.24	0.24	0.12	_	_	_		35
				12	25'	0.29	0.38	0.31	0.12	_	_	_		35
					30'	0.43	0.46	0.47	0.12	_	_	_		35
					0.33' - <2'	0.24	0.35	0.34	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.22	0.35	0.34	0.12	-	-	-		45
				7	3' - <5'	0.15	0.24	0.24	0.12	_	_	_		45
5' x 4'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	_	_	_		36
					15'	0.22	0.25	0.27	0.12	_	_	_		35
				12	20'	0.29	0.33	0.34	0.12	-	-	_		35
					25'	0.36	0.41	0.43	0.12	-	-	-		35
					30'	0.42	0.50	0.51	0.12	-	-	-		35
					0.33' - <2'	0.24	0.37	0.36	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.21	0.37	0.36	0.12	-	-	-		45
					3' - <5'	0.16	0.24	0.25	0.12	-	-	-		45
5' x 5'	10	10	10	to	5' - 10'	0.17	0.24	0.24	0.12	-	-	-		45
					15'	0.24	0.27	0.29	0.12	-	-	-		36
				12	20'	0.30	0.36	0.38	0.12	-	-	-		35
					25'	0.37	0.44	0.47	0.12	-	-	-		35
					30'	0.44	0.53	0.56	0.12	-	-	-		35
					0.33' - <2'	0.28	0.40	0.36	0.24	0.24	0.24	0.28	5	_
				4	2' - <3'	0.28	0.40	0.36	0.12	-	-	-	Note	43
					3' - <5'	0.22	0.26	0.28	0.12	-	-	-	Nc	39
6' x 3'	10	10	10	to	5' - 10'	0.24	0.24	0.24	0.12	-	-	-	ral	39
					15'	0.34	0.31	0.32	0.12	-	-	-	General	38
				12	20'	0.44	0.41	0.42	0.12	-	-	_		38
					25'	0.54	0.52	0.53	0.12	-	-	-	See	38
					30'	0.64	0.63	0.64	0.12	-	-	-	S	38
					0.33' - <2'	0.27	0.42	0.39	0.24	0.24	0.24	0.27		_
				4	2' - <3'	0.27	0.42	0.39	0.12	-	-	-		43
<i>a</i> , <i>a</i> ,					3' - <5'	0.21	0.28	0.30	0.12	-	-	-		39
6' x 4'	10	10	10	to	5' - 10'	0.23	0.24	0.25	0.12	_	_	-		39
					15'	0.32	0.34	0.35	0.12	-	-	-		38
				12	20'	0.42	0.45	0.47	0.12	-	=	-		38
					25' 30'	0.51	0.56	0.58	0.12	-	-	-		38
						0.61	0.68	0.70	0.12	- 0.24	- 0.24	- 0.26		38
					0.33' - <2'	0.26	0.44	0.42	0.24	0.24	0.24	0.26		12
				4	2' - <3' 3' - <5'	0.26	0.44	0.42	0.12	_	-	-		43
6' x 5'	10	10	10		5' - 10'	0.22 0.24	0.30 0.25	0.33 0.27	0.12	_		_		39
0 x 3	10	10	10	to	3 - 10 15'	0.24	0.25	0.27	0.12	_	-	_		38
				12	20'	0.33	0.36	0.59	0.12	_	_	_		38
				12	25'	0.42	0.48	0.63	0.12	_	_	_		38
					30'	0.52	0.74	0.76	0.12	_	_	_		38
					0.33' - <2'	0.01	0.74	0.44	0.12	0.24	0.24	0.27		-
					2' - <3'	0.27	0.46	0.44	0.24	-	-	-		52
				4	3' - <5'	0.27	0.40	0.44	0.12	_	_	_		52
6' x 6'	10	10	10	t 0	5' - 10'	0.25	0.27	0.30	0.12	_		_		43
5 A 0		10	10	to	15'	0.25	0.39	0.42	0.12	_	_	_		39
				12	20'	0.45	0.52	0.55	0.12	_	_	_		39
				12	25'	0.43	0.65	0.68	0.12	_	_	_		38
				1		U.J.	0.00	0.00	U.12	I		1	i	, 50

LAST REVISION 07/01/13

≥ DESCRIPTION:

FDOT

TABL	E 11A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT	DESIC	SNS (.	3" CO	VER)	- 7' 5	SPAN:	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH					(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.42	0.58	0.52	0.22	0.22	0.31	0.42		-
				4	2' - <3'	0.42	0.58	0.51	0.11	-	-	-		43
					3' - <5'	0.36	0.41	0.44	0.11	-	_	-		43
7' x 4'	9	9	9	to	5' - 10'	0.39	0.40	0.39	0.11	-	-	-		43
					15'	0.56	0.56	0.58	0.11	-	-	-		41
				12	20'	0.74	0.76	0.77	0.11	-	-	-		41
					25'	0.92	0.97	0.97	0.11	-	_	-		41
	9	9.5	9	7 to 12	30'	1.09	1.18	1.10	0.11	-	-	-		41
					0.33' - <2'	0.41	0.61	0.55	0.22	0.23	0.30	0.41		-
				4	2' - <3'	0.41	0.61	0.55	0.11	-	-	-		47
					3' - <5'	0.37	0.43	0.47	0.11	-	-	-		43
7' x 5'	9	9	9	to	5' - 10'	0.39	0.41	0.43	0.11	-	-	-		43
					15'	0.56	0.61	0.63	0.11	-	-	-	5	41
				12	20'	0.73	0.82	0.83	0.11	-	-	-	Note	41
					25'	0.90	1.04	1.06	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.06	1.26	1.19	0.11	-	-	-	General	41
					0.33' - <2'	0.42	0.63	0.58	0.22	0.24	0.30	0.42	ne	-
				4	2' - <3'	0.42	0.63	0.58	0.11	_	-	-		59
					3' - <5'	0.38	0.45	0.50	0.11	-	-	-	See	47
7' x 6'	9	9	9	to	5' - 10'	0.41	0.44	0.47	0.11	-	-	-	5	43
					15'	0.57	0.65	0.68	0.11	_	-	-		41
				12	20'	0.75	0.87	0.90	0.11	-	-	-		41
					25'	0.93	1.11	1.13	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.07	1.35	1.27	0.11	-	-	-		41
					0.33' - <2'	0.44	0.66	0.61	0.22	0.25	0.31	0.44		
				4	2' - <3'	0.44	0.65	0.61	0.11	-	-	-		59
					3' - <5'	0.41	0.47	0.52	0.11	-	-	-		59
7' x 7'	9	9	9	to	5' - 10'	0.44	0.47	0.52	0.11	-	-	-		47
					15'	0.62	0.69	0.74	0.11	_	-	-		43
				12	20'	0.80	0.93	0.97	0.11	-	-	-		43
					25 ^t	0.99	1.18	1.22	0.11	-	-	-		43
	9	9.5	9	7 to 12	30'	1.12	1.43	1.36	0.11	_	-	-		41

TABI	LE 11B	- ST	ANDA	ARD PRI	ECAST BOX	CULV	ERT	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPANS	5
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH					(5	q. in./F	t.)			LENGTH
(=,)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.33	0.49	0.44	0.24	0.24	0.24	0.33		-
				4	2' - <3'	0.33	0.49	0.44	0.12	-	-	-		43
					3' - <5'	0.29	0.35	0.38	0.12	-	-	-		43
7' x 4'	10	10	10	to	5' - 10'	0.31	0.30	0.31	0.12	-	-	-		43
					15'	0.44	0.44	0.45	0.12	-	-	-		41
				12	20'	0.58	0.59	0.60	0.12	-	-	-		41
					25'	0.71	0.74	0.75	0.12	-	-	-		41
					30'	0.85	0.91	0.91	0.12	-	-	-		41
					0.33' - <2'	0.32	0.51	0.47	0.24	0.24	0.24	0.32		-
				4	2' - <3'	0.32	0.51	0.47	0.12	-	-	-		47
					3' - <5'	0.29	0.37	0.41	0.12	-	-	-		43
7' x 5'	10	10	10	to	5' - 10'	0.31	0.32	0.35	0.12	-	-	-		43
					15'	0.44	0.47	0.50	0.12	-	-	-	5	41
				12	20'	0.57	0.63	0.65	0.12	-	-	-	Note	41
					25'	0.70	0.80	0.82	0.12	-	-	-	No	41
					30'	0.84	0.97	0.99	0.12	-	-	-	le.	41
					0.33' - <2'	0.33	0.53	0.50	0.24	0.24	0.24	0.33	General	-
				4	2' - <3'	0.33	0.53	0.50	0.12	-	-	-	<i>Ge</i>	59
					3' - <5'	0.30	0.38	0.43	0.12	-	-	-	ee	47
7' x 6'	10	10	10	to	5' - 10'	0.33	0.35	0.38	0.12	-	-	-	Š	43
					15'	0.45	0.51	0.54	0.12	-	_	-		41
				12	20'	0.58	0.68	0.70	0.12	-	-	-		41
					25'	0.72	0.85	0.88	0.12	-	-	-		41
					30'	0.85	1.04	1.06	0.12	-	-	-		41
					0.33' - <2'	0.35	0.55	0.52	0.24	0.24	0.24	0.35		-
				4	2' - <3'	0.35	0.55	0.52	0.12	-	-	-		59
					3' - <5'	0.32	0.40	0.46	0.12	-	-	-		59
7' x 7'	10	10	10	to	5' - 10'	0.35	0.37	0.41	0.12	-	-	-		47
					15'	0.48	0.54	0.58	0.12	-	-	-		43
				12	20'	0.62	0.72	0.76	0.12	-	-	-		43
					25'	0.76	0.90	0.94	0.12	-	-	-		43
					30'	0.90	1.10	1.13	0.12	_	_	_		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: LAST REVISION 07/01/13



SPAN x RISE	SLAE	3 / WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EX
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGT
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			1	T		1	1	1	(M) (in.)
(FL.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	As9	(111.)
					0.33' - <2'	0.42	0.56	0.49	0.24	0.24	0.32	0.41		
				4	2' - <3'	0.42	0.56	0.49	0.12	-	-	-		50
					3' - <5'	0.38	0.42	0.46	0.12	-	-	-		50
8' x 4'	10	10	10	to	5' - 10'	0.41	0.38	0.39	0.12	-	-	-	-	45
					15'	0.59	0.56	0.57	0.12	-	-	-		41
				12	20'	0.78	0.75	0.76	0.12	-	-	-	-	41
	10	10.5	1.0	0 . 10	25'	0.97	0.96	0.96	0.12	_	_	-		41
	10	10.5	10	8 to 12	30'	1.15	1.16	1.10	0.12	-	-	-		41
					0.33' - <2'	0.40	0.58	0.52	0.24	.034	0.31	0.40	-	-
				4	2' - <3'	0.40	0.58	0.52	0.12	-	-	-		50
01 51	1.0	10	1.0		3' - <5' 5' - 10'	0.37	0.45	0.48	0.12	-	_	-		45 45
8' x 5'	10	10	10	to	15' - 10'	0.41	0.41	0.43		-	_	-		45
				1.2	20'	0.58	0.60	0.62	0.12	_	_	_		41
				12	25'	0.76 0.94	0.81 1.03	0.81	0.12	_	_	_	-	41
	10	10.5	10	8 to 12	30'	1.10	1.24	1.24	0.12	_	_	_		41
	10	10.5	10	0 10 12	0.33' - <2'	0.40	0.60	0.55	0.12	0.24	0.30	0.40	5	-
				1	2' - <3'	0.40	0.60	0.55	0.12	-	-	-	l .	50
				4	3' - <5'	0.40	0.47	0.53	0.12	_	_	_	Note	50
8' x 6'	10	10	10	to	5' - 10'	0.42	0.43	0.46	0.12	_	_	_		45
0 10	10	10	10	1 10	15'	0.58	0.64	0.67	0.12	_	_	_	erë	41
				12	20'	0.76	0.86	0.88	0.12	_	_	_	General	41
				12	25'	0.94	1.09	1.11	0.12	_	_	_		41
	10	10.5	10	8 to 12	30'	1.09	1.32	1.26	0.12	_	_	_	See	41
		10.0			0.33' - <2'	0.41	0.63	0.58	0.24	0.24	0.30	0.41	1	_
				4	2' - <3'	0.41	0.63	0.58	0.12	_	_	_	-	55
				~	3' - <5'	0.39	0.49	0.53	0.12	_	_	_	-	55
8' x 7'	10	10	10	to	5' - 10'	0.44	0.46	0.50	0.12	-	-	-		50
					15'	0.61	0.68	0.72	0.12	-	-	_		45
				12	20'	0.78	0.91	0.94	0.12	-	_	-		41
					25'	0.97	1.16	1.18	0.12	-	-	-	1	41
	10	10.5	10	8 to 12	30'	1.11	1.40	1.34	0.12	-	-	-		41
					0.33' - <2'	0.44	0.64	0.60	0.24	0.24	0.31	0.44		-
				4	2' - <3'	0.44	0.64	0.60	0.12	-	-	-		65
					3' - <5'	0.42	0.51	0.56	0.12	-	-	-		65
8' x 8'	10	10	10	to	5' - 10'	0.47	0.50	0.55	0.12	-	_	-		55
					15'	0.65	0.72	0.77	0.12	-	-	-		45
				12	20'	0.84	0.96	1.01	0.12	-	=	-		45
					25'	1.03	1.22	1.26	0.12	-	_	-		41
	10	10.5	10	8 to 12	30'	1.16	1.47	1.42	0.12		-	_		41

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

DESCRIPTION:

FDOT

SLAB / WALL THICKNESS

(Tb)

(in.)

BOT. | SIDE | HAUNCH

(H)

(in.)

(Tw)

(in.)

TOP

(Tt)

(in.)

(Ft.)

TABLE 13A - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 9' SPANS

As2

EARTH COVER

ABOVE

TOP SLAB

TABL	.E 13E	3 - ST	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESI	GNS (3" CC	VER)	- 9' 5	SPAN	S
SPAN x RISE	SLAE	/ WAL	L THIC		DESIGN			R			T AREA	15		As1 EXT
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(54.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE				1	1				(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	A54	As5	As7	As8	A59	(in.)
				1	0.33' - <2'	0.49	0.65	0.57	0.24	0.24	0.40	0.48		-
				4	2' - <3'	0.49	0.65	0.57	0.12	-	-	-		54
	10	1.0	10	+ 0	3' - <5'	0.46	0.54	0.53	0.12	-	-	-		49
9' x 5'	10	10	10	to	5' - 10'	0.52	0.50	0.51	0.12	-	-	-		49
				12	15'	0.75	0.74	0.75	0.12	-	-	-		44
				12	20'	0.98	1.01	1.00	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.21	1.27	1.19	0.12	-	-	-		44
	11	11.5	10	12	30'	1.30	1.36	1.30	0.12	-	-	-		44
					0.33' - <2'	0.48	0.68	0.60	0.24	0.24	0.39	0.48		-
				4	2' - <3'	0.48	0.68	0.60	0.12	-	-	-		54
	1.0	10	1.0		3' - <5'	0.45	0.57	0.56	0.12	-	-	-		49
9' x 6'	10	10	10	to	5' - 10'	0.52	0.53	0.56	0.12	-	-	-		49
				12	15'	0.74	0.79	0.81	0.12	-	-	-		44
				12	20'	0.97	1.07	1.07	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.18	1.35	1.28	0.12	-	-	-		44
	11	11.5	10	12	30'	1.27	1.44	1.38	0.12	-	-	-		44
					0.33' - <2'	0.49	0.70	0.63	0.24	0.24	0.39	0.49	5	-
				4	2' - <3'	0.49	0.70	0.63	0.12	-	-	-	Note	59
	1.0	1.0	1.0	,	3' - <5'	0.46	0.59	0.59	0.12	-	-	-		54
9' x 7'	10	10	10	to	5' - 10'	0.54	0.57	0.60	0.12	-	-	-	General	49
				4.0	15'	0.75	0.84	0.86	0.12	-	-	-	neı	44
				12	20'	0.98	1.13	1.14	0.12	-	-	-	<i>6e</i>	44
	10	10.5	10	8 to	25'	1.18	1.43	1.36	0.12	-	-	-	See	44
	11	11.5	10	12	30'	1.28	1.52	1.46	0.12	-	-	-	Sé	44
					0.33' - <2'	0.51	0.72	0.65	0.24	0.24	0.39	0.51		-
				4	2' - <3'	0.51	0.72	0.65	0.12	-	-	-		59
	1 , .	1.0	1		3' - <5'	0.49	0.61	0.62	0.12	-	-	-		59
9' x 8'	10	10	10	to	5' - 10'	0.57	0.60	0.65	0.12	-	-	-		54
				1.0	15'	0.79	0.89	0.92	0.12	-	-	-		44
				12	20'	1.02	1.20	1.22	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.21	1.50	1.44	0.12	-	-	-		44
	11	11.5	10	12	30'	1.33	1.59	1.54	0.12	-	-	-		44
					0.33' - <2'	0.54	0.74	0.68	0.24	0.24	0.41	0.54		
				4	2' - <3'	0.54	0.74	0.68	0.15	-	-	-		72
				l .	3' - <5'	0.53	0.63	0.64	0.13	-	-	_		72
9' x 9'	10	10	10	to	5' - 10'	0.62	0.64	0.70	0.12	-	-	-		59
					15'	0.85	0.94	0.99	0.12	-	-	-		49
				12	20'	1.09	1.26	1.29	0.12	-	-	-		49
	10	10.5	10	8 to	25'	1.28	1.56	1.52	0.12	-	-	-		44
	11	11.5	10	12	30'	1.42	1.66	1.66	0.12	-	-	-		44

NOTES:

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

DESCRIPTION:



As1 EXT

LENGTH

(M)

(in.)

54

49

49

44

44

44

44

54

49

49

44

44

44

44

59

54

49

44

44

44

44

59

59

54

44

44

44

44

72

72

59 49

49

44

44

REINFORCEMENT AREAS

(sq. in./Ft.)

As7 | As8 | As9

0.61

0.60

0.61

0.53

0.57

0.52

0.51

0.51

0.52

0.55

As3 | As4 | As5 |

(S) (R) TOP BOT. SIDE HAUNCH EARTH COVER (sq. in./Ft.) LENGTH														
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH					(5	q. in./F	t.)			
	(Tt)		' '	' '										1 ' '
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.60	0.66	0.54	0.27	0.27	0.52	0.56		-
				4	2' - <3'	0.60	0.66	0.54	0.14	-	-	-		62
				to	3' - <5'	0.60	0.61	0.53	0.14	-	_	-		62
11' x 4'	11	11	11	10	5' - 10'	0.79	0.63	0.62	0.14	_	_	-		55
				12	15'	1.01	0.82	0.79	0.14	-	-	-		55
				12	20'	1.34	1.11	1.06	0.14	-	-	-		55
	12	12	11	8 to	25'	1.52	1.27	1.23	0.14	-	-	-		55
	13.5	13.5	11	12	30'	1.54	1.37	1.34	0.14	-	-	-		50
					0.33' - <2'	0.57	0.71	0.60	0.27	0.27	0.47	0.53		-
				4	2' - <3'	0.56	0.71	0.60	0.14	-	_	-		62
				to	3' - <5'	0.56	0.67	0.59	0.14	-	-	-		55
11' x 6'	11	11	11	12	5' - 10'	0.73	0.71	0.72	0.14	-	-	-		55
					15'	0.92	0.92	0.91	0.14	-	-	-		50
	11	11	11	8	20'	1.21	1.25	1.21	0.14	-	-	-		50
	12	12	11	to	25'	1.37	1.43	1.39	0.14	-	-	-		50
	13.5	13.5	11	12	30'	1.39	1.53	1.50	0.14	-	-	-		50
					0.33' - <2'	0.55	0.76	0.66	0.27	0.27	0.46	0.55	5	-
				4	2' - <3'	0.55	0.76	0.66	0.14	-	_	-	Note	62
				to	3' - <5'	0.54	0.72	0.65	0.14	-	-	-	N _O	62
11' x 8'	11	11	11	12	5' - 10'	0.73	0.79	0.82	0.14	-	-	-	le.	55
					15'	0.93	1.03	1.03	0.14	-	_	-	General	50
	11	11	11	8	20'	1.21	1.39	1.36	0.14	-	_	-	(ee	50
	12	12.5	11	to	25'	1.34	1.56	1.50	0.14	-	-	-	See	50
	13.5	13.5	11	12	30'	1.41	1.66	1.65	0.14	-	-	-	Se	50
					0.33' - <2'	0.60	0.81	0.71	0.27	0.27	0.48	0.60		-
				4	2' - <3'	0.60	0.81	0.71	0.15	_	-	-		75
				to	3' - <5'	0.61	0.77	0.70	0.14	-	-	-		69
11' × 10'	11	11	11	12	5' - 10'	0.80	0.88	0.93	0.14	-	-	-		62
					15'	1.01	1.13	1.15	0.14	_	-	-		55
[11	11	11	8	20'	1.30	1.52	1.52	0.14	-	-	-		50
[12	12.5	11	to	25 [']	1.42	1.70	1.65	0.14	-	-	-		50
	13.5	14	11	12	30'	1.53	1.77	1.74	0.14	-	-	-		50
					0.33' - <2'	0.64	0.83	0.74	0.27	0.27	0.51	0.64		_
				4	2' - <3'	0.64	0.83	0.74	0.21	-	-	-		86
				to	3' - <5'	0.67	0.79	0.75	0.21	_	_	-		75
11' × 11'	11	11	11	12	5' - 10'	0.88	0.93	0.99	0.19	-	-	-		69
					15'	1.09	1.19	1.23	0.16	-	-	-		55
	11	11	11	8	20'	1.40	1.59	1.60	0.15	_	-	_		55
	12	12.5	11	to	25'	1.54	1.77	1.73	0.15	-	-	-		50
	13.5	14	11.5	12	30'	1.57	1.77	1.76	0.14	_	_	_		50

NOTES:

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

LAST REVISION 07/01/13

DESCRIPTION:

FDOT

2
1:53:10
202

12' x 4'					J - 10	0.01	0.01	0.01	0.15	_	_	_		00
12 X 4				1.2	15'	1.04	0.80	0.77	0.15	-	-	-		59
				12	20'	1.37	1.08	1.03	0.15	-	-	-		59
	13	13	12	8 to	<i>25</i> ′	1.58	1.26	1.21	0.15	-	-	-		59
	14.5	14.5	12	12	30'	1.63	1.38	1.34	0.15	-	_	-		53
				4	0.33' - <2'	0.56	0.70	0.57	029	0.29	0.47	0.52		_
				4	2' - <3'	0.56	0.70	0.57	0.15	-	-	-		66
	12	12	12	to	3' - <5'	0.56	0.67	0.57	0.15	-	-	-		59
12' x 6'	12	12	12	10	5' - 10'	0.74	0.69	0.70	0.15	-	-	-		59
12 % 0				12	15'	0.94	0.90	0.88	0.15	-	-	-		53
					20'	1.23	1.22	1.17	0.15	-	-	-		53
	13	13	12	8 to	25'	1.40	1.42	1.37	0.15	-	-	-		53
	14.5	15	12	12	30'	1.44	1.54	1.48	0.15	-	-	-		53
					0.33' - <2'	0.55	0.75	0.63	0.29	0.29	0.45	0.53	5	-
				4	2' - <3'	0.55	0.75	0.63	0.15	-	-	-	ote	66
	12	12	12	to	3' - <5'	0.55	0.73	0.63	0.15	-	-	-	ž	59
12' x 8'	1	12	'-	12	5' - 10'	0.73	0.77	0.79	0.15	-	-	-	ral	59
12 / 0					15'	0.93	1.00	0.99	0.15	-	-	-	General Note	53
	12	12	12	8	20'	1.21	1.35	1.31	0.15	-	-	-		53
	13	13.5	12	to	25'	1.35	1.55	1.48	0.15	-	-	-	See	53
	14.5	15	12	12	30'	1.40	1.67	1.62	0.15	-	-	-	S	53
					0.33' - <2'	0.57	0.80	0.68	0.29	0.29	0.46	0.57		-
				4	2' - <3'	0.57	0.80	0.68	0.15	-	-	-		73
	12	12	12	to	3' - <5'	0.59	0.77	0.68	0.15	-	-	-		66
12' x 10'				12	5' - 10'	0.78	0.85	0.89	0.15	-	-	-		59
					15'	0.98	1.10	1.11	0.15	-	-	-		53
	12	12	12	8	20'	1.26	1.47	1.45	0.15	-	-	-		53
	13	13.5	12	to	25'	1.39	1.68	1.63	0.15	-	-	-		53
	14.5	15	12	12	30'	1.48	1.79	1.76	0.15	-	-	-		53
				_	0.33' - <2'	0.65	0.84	0.73	0.29	0.29	0.50	0.65		_
				4	2' - <3'	0.65	0.84	0.73	0.23	-	-	-		93
	12	12	12	to	3' - <5'	0.68	0.81	0.75	0.22	-	-	-		80
12' x 12'				12	5' - 10'	0.90	0.94	1.01	0.21	-	-	-		73
					15'	1.12	1.20	1.24	0.18	-	-	-		59
	12	12	12	8	20'	1.42	1.60	1.61	0.16	-	-	-		59
	13	13.5	12	to	25'	1.57	1.81	1.78	0.16	-	-	-		53
	14.5	15	12.5	12	30'	1.63	1.86	1.85	0.15	-	-	-		53

TABLE 16 - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 12' SPANS

As1

0.59

0.60

0.60

0.81

As2

0.64

0.64

0.61

0.61

As3

0.51

0.51

0.51

0.61

REINFORCEMENT AREAS

(sq. in./Ft.)

0.29

0.52

As7 | As8 | As9

0.55

As4 As5

0.29

0.15

0.15

0.15

LENGTH

(in.)

73

66

66

DESIGN

EARTH COVER

ABOVE

TOP SLAB

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

NOTES:

SLAB / WALL THICKNESS

(Tb)

(in.)

12

BOT. | SIDE | HAUNCH

(H)

(in.)

to

(Tw)

(in.)

12

SPAN x RISE

(Ft.)

12' x 4'

(R)

TOP

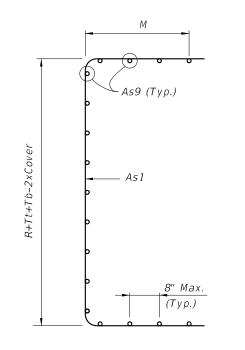
(Tt)

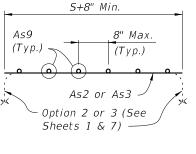
(in.)

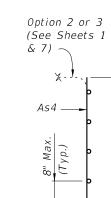
12

- 1. See Sheet 2 of 14 for General Notes.
- 2. See Sheet 7 of 14 for Reinforcing Details and dimension locations.

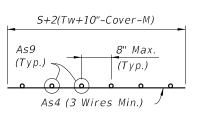
WELDED WIRE REINFORCEMENT BENDING DIAGRAM

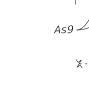






WWR PIECE NO. 2 (2 Reqd. per segment)



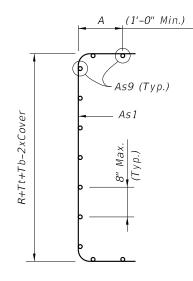


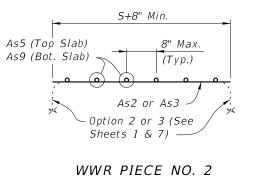
WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (Tongue Reinforcement) (4 Reqd. per segment)

WWR PIECE NO. 3 (2 Reqd. per segment)

TYPE 2 BOX SECTION (DESIGN EARTH COVER 2' OR GREATER)



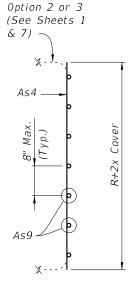


(2 Reqd. per segment)

S+2(Tw+10"-Cover-A)

8" Max.
(Typ.)

As7 (Top Slab)
As8 (Bot. Slab)



WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (2 Regd. per segment) WWR PIECE NO. 3 (2 Reqd. per segment)

TYPE 1 BOX SECTION (DESIGN EARTH COVER LESS THAN 2')

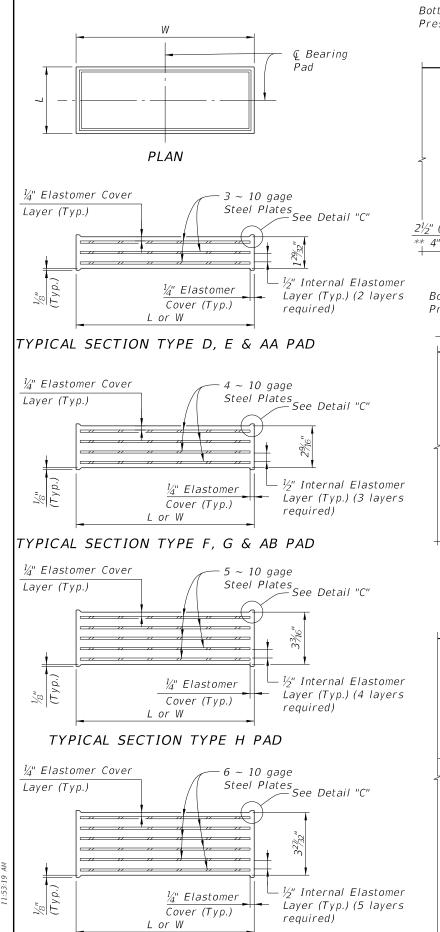
REINFORCEMENT NOTES:

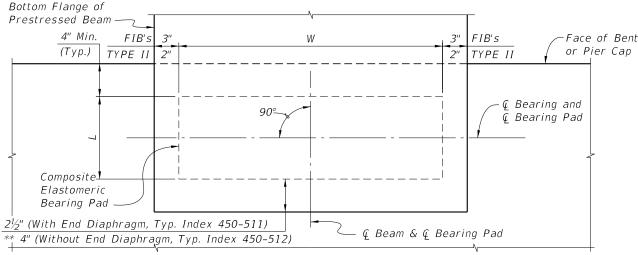
- 1. Reinforcement bending dimensions are out-to-out.
- 2. See General Notes 4, 5 and 6 on Sheet 2.
- 3. See Tables 1 thru 16 for dimensions M, R, S, Tb, Tt and Tw.
- 4. Dimension "A" is determined by the Manufacturer in accordance with the requirements of Detail "B" on Sheets 1 and 7.

LAST O DESCRIPTION:

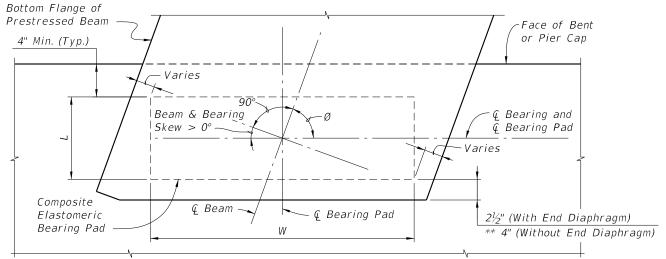
07/01/13



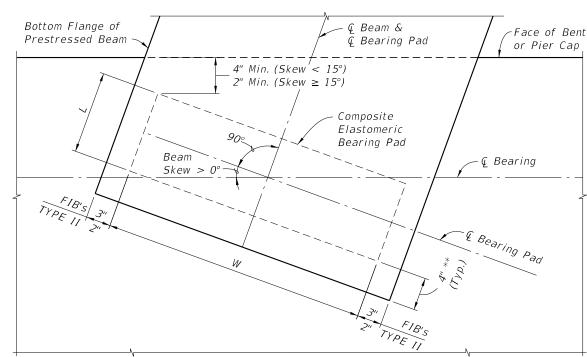




PARTIAL PLAN (Beam & Bearing Skew = 0°)



PARTIAL PLAN (Beam & Bearing Skew > 0°) (Use Index 450-511)

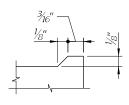


PARTIAL PLAN WITH SQUARED END BEAM (Use Index 450-512)

(Beam Skew > 0° ; Bearing Skew = 0°)

*BEVELED BEARING PLATE BEAMBEARING PAD TYPE**DIMENSIONS** DIMENSIONS PAD TYPE D (See Note 1) D 2'-8" 3'-0" 8" 1'-0" (G=110psi) 10" 3'-0" 2'-8" 1'-0" (G=110psi) F 2'-8" 3'-0" 10" 1'-0" I-BE (G=110psi) G 3'-0" 10" 2'-8" 1'-0" (G=150psi) Н 3'-0" 10" 2'-8" 1'-0" (G=150psi) 3'-0" 10" 2'-8" 1'-0" (G=150psi) 3'-0" 1'-0" 2'-8" 1'-11/2" (G=150psi) AA0. 10" 1'-2" 1'-0" 1'-4" (G=110psi) AASHT TYPE AΒ 1'-2" 10" 1'-0" 1'-4" (G=150psi)

- * Work this sheet with the appropriate type Bearing Plate Detail (See Bearing Plate Data Table) and BEARING PAD DATA TABLE in the Structures Plans. See TABLE OF BEAM VARIABLES and BEARING PLATE DATA TABLE in the Structures Plans for locations where beveled bearing plates are required.
- ** Offset to End of Beam is reduced to 2" for Type K Pad.



DETAIL "C"

BEARING PAD NOTES:

- 1. Neoprene in Type D, E, F & AA bearing pads shall have a shear modulus (G) of 110 psi. Neoprene in Type G, H, J, K & AB bearing pads shall have a shear modulus (G) of 150 psi.
- 2. Steel Plates in bearing pads shall conform to ASTM A1011 Grade 36, Type 1.
- 3. See Bearing Pad Data Table in Structures Plans for quantities of Type D, E, F, G, H, J, K, AA and/or AB Bearing Pads.

FDOT

FY 2024-25

COMPOSITE ELASTOMERIC BEARING PADS -PRESTRESSED FLORIDA-I & AASHTO TYPE II BEAM INDEX

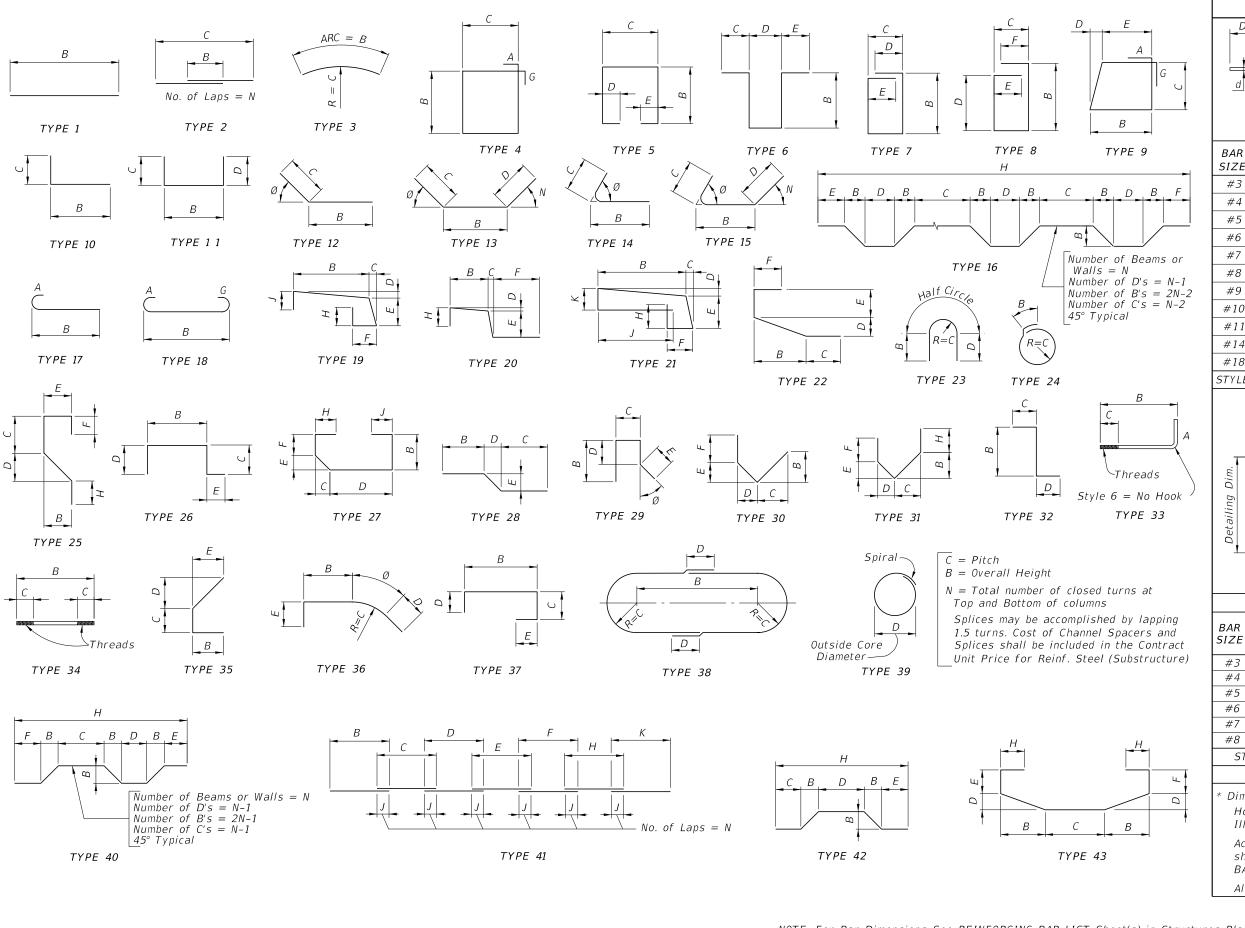
SHEET

1 of 1 400-510

REVISION

07/01/15

TYPICAL SECTION TYPE J & K PAD



		ноок Е	DETAILS									
Dei	Detailing Dim. Hook A or G A or G Detailing Dim. Hook A or G											
	180°			90°								
BAR	D	180° H	OOKS	90° HOOKS								
SIZE		A OR G	J	A OR G								
#3	21/4"	5"	3"	6"								
#4	3"	6"	4"	8"								
#5	3¾"	7"	5"	10"								
#6	4½"	8"	6"	1'-0"								

10"

11"

1'-3"

1'-5"

7"

8"

11¾"

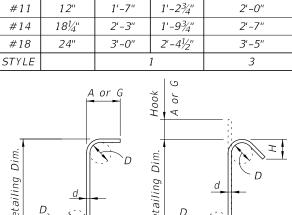
1'-2"

1'-4"

1'-7"

1'-10"

51/4"



STIRRUPS (TIES SIMILAR)

STIRRUP & TIE HOOK DIMENSIONS

135°

	STIRRUP & TIE HOUR DIMENSIONS						
BAR	D	90° HOOKS	135° HOOKS				
SIZE		A or G	A or G	н *			
#3	1½"	4"	4"	2½" 3"			
#4	2"	4½"	4½"	3"			
#5	2½"	6"	5½"	3 ³ / ₄ " 4 ¹ / ₂ "			
#6	4½"	1'-0"	8"				
#7	51/4"	1'-2"	9"	5½"			
#8	6"	1'-4"	10½"	6"			
STYLE		4	5				

STYLE 6 = NO HOOK

Dimension is approximate.

Hook Styles Detailed on this sheet are for Illustration Only.

Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.

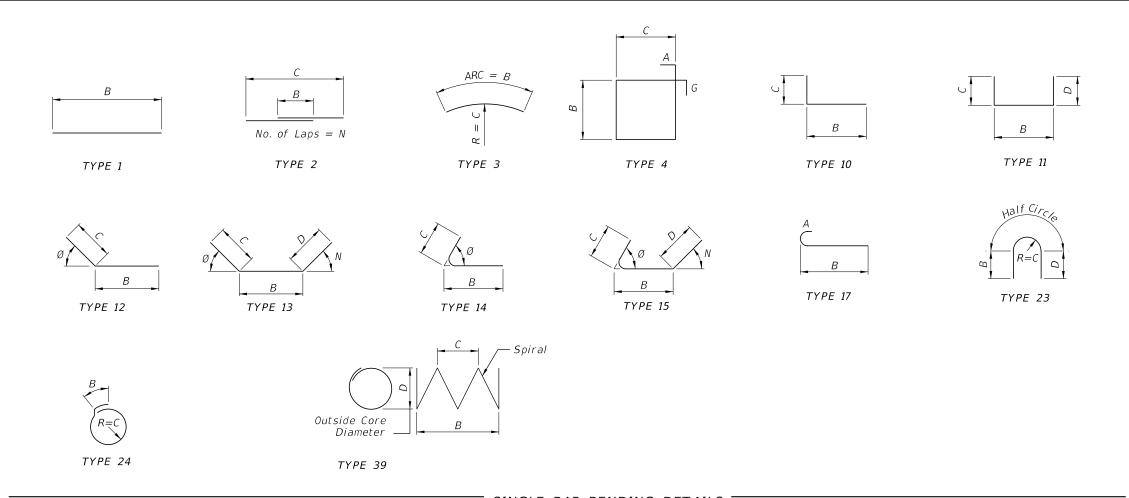
All Dimensions are out-to-out.

NOTE: For Bar Dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

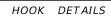
REVISION 11/01/20

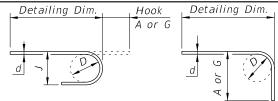
DESCRIPTION:





SINGLE BAR BENDING DETAILS =





180°

BAR	D	180° HOOKS		90° HOOKS		
SIZE		A OR G	J	A OR G		
#3	2½"	5"	3"	6"		
#4	3"	6"	4"	8"		
#5	3¾"	7"	5"	10"		
#6	4½"	8"	6"	1'-0''		
#7	5½"	10"	7"	1'-2"		
#8	6"	11"	8"	1'-4"		
STYLE			1	3		

NOTES

GENERAL

All dimensions are out-to-out.

For Bar dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

SPIRALS (TYPE 39 BARS)

C = Pitch

B = Overall Height

N = Total number of closed turns at

Top and Bottom of columns

Spirals = 1.5 turns

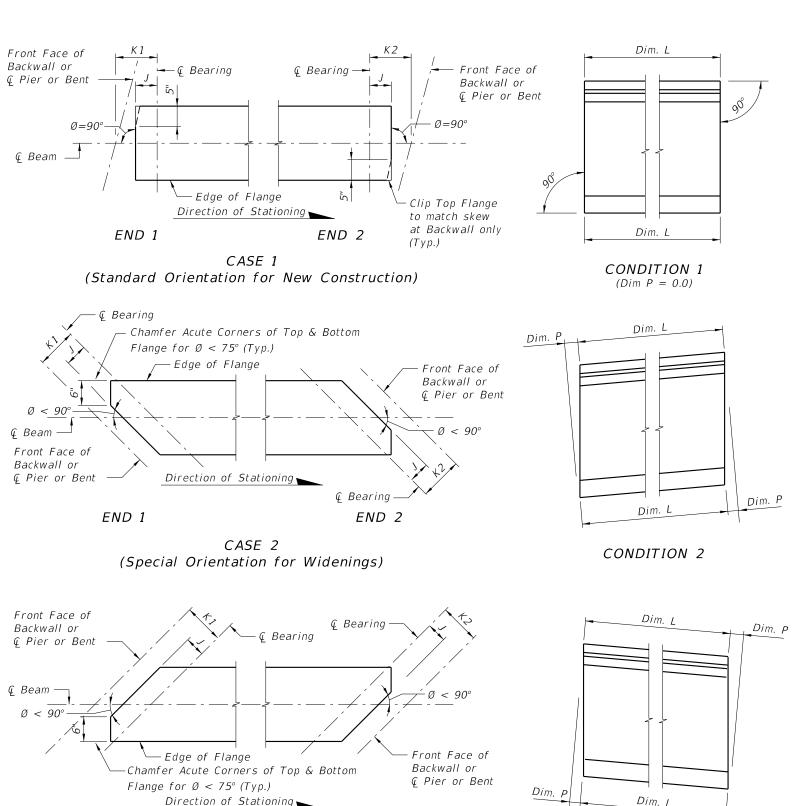
Include spiral splice in Contract Unit Price for FRP Reinforcing.

H00K5

All Dimensions are approximate.

Hook Styles Detailed on this sheet are for Illustration Only. Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.

DESCRIPTION:



END 2

BEAM NOTES

- 1. Work this Index with the Florida-I Beam Standard Details (Index 450-036 thru 450-096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum.
- Stress Strands N to 10 kips each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
 - A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands.
 - B. For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
 - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
 - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
 - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.
 - D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement.
- 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
 - B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
- A. The superstructure environmental classification is slightly or moderately aggressive
- Clear cover to adjacent steel reinforcing is 1"or greater
- Hole inside diameter is 2" maximum
- Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

END 1

DESCRIPTION:

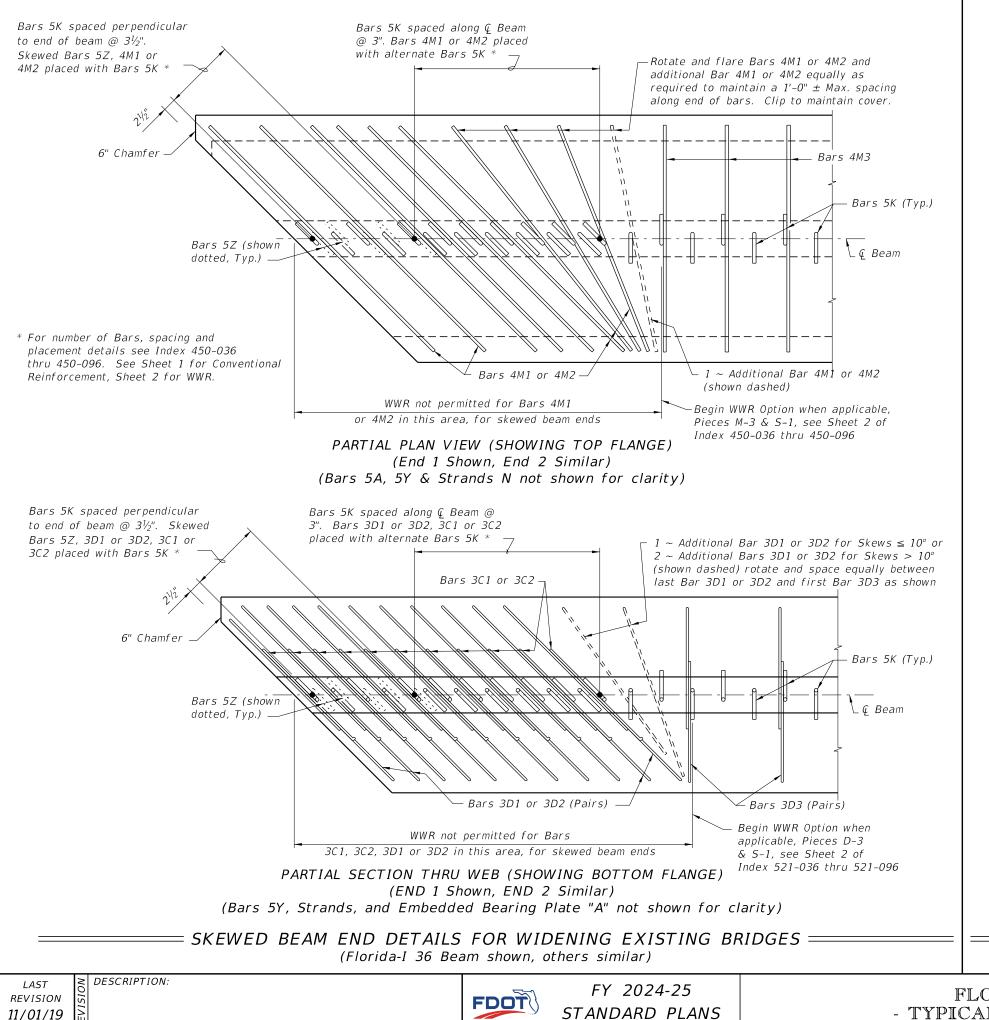
CASE 3

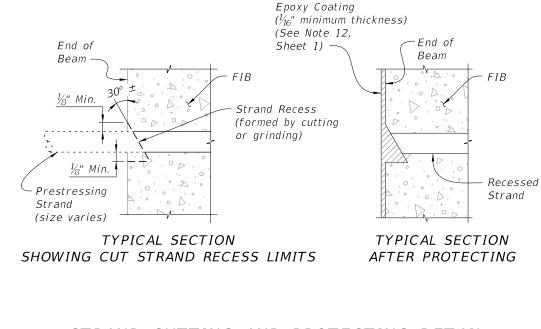
(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

CONDITION 3

SCHEMATIC END ELEVATIONS OF BEAMS (Showing Vertical Bevel of Beam End)





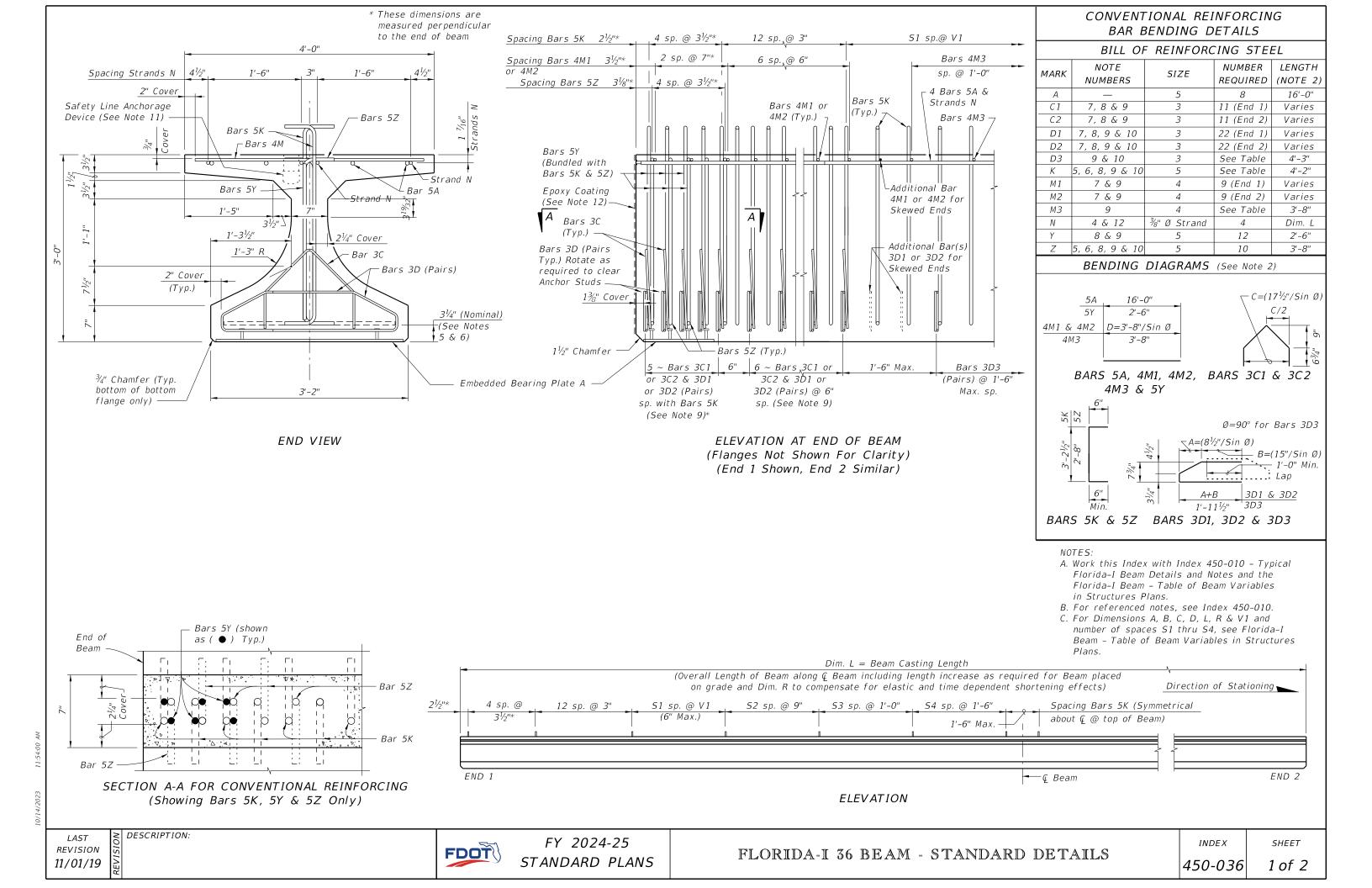
==== STRAND CUTTING AND PROTECTING DETAIL ====

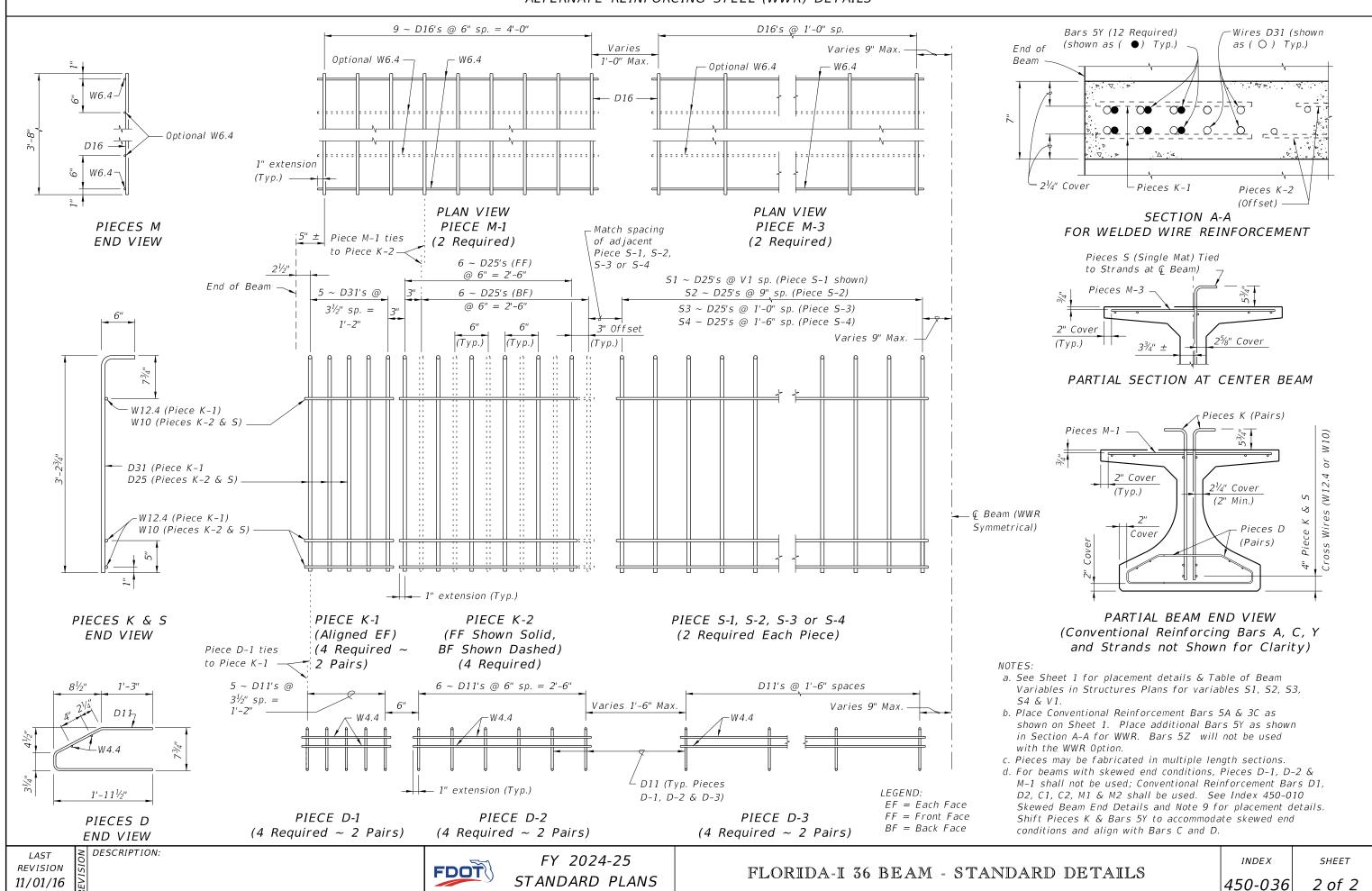
11/01/19

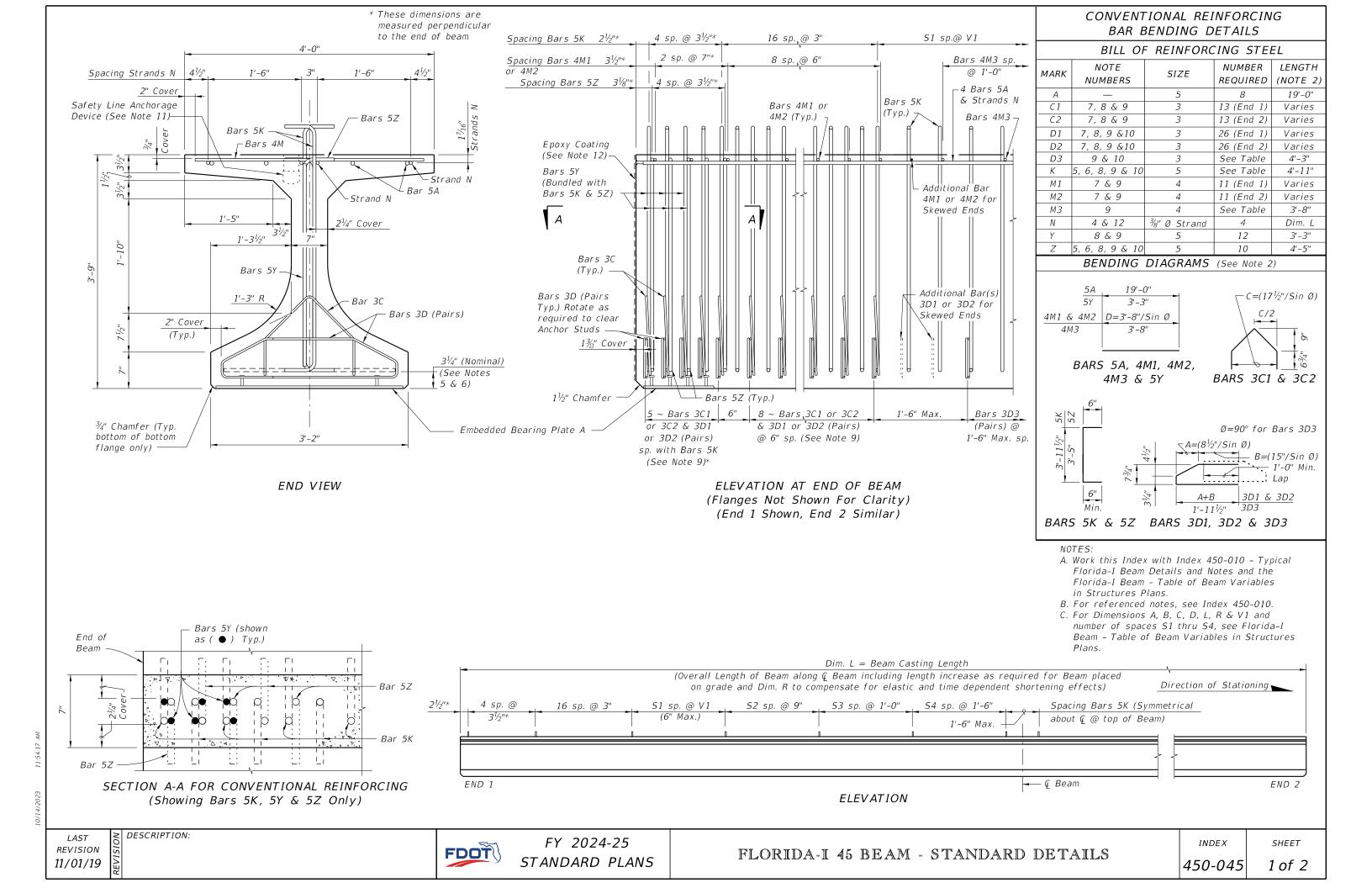
FLORIDA-I BEAM - TYPICAL DETAILS & NOTES INDEX

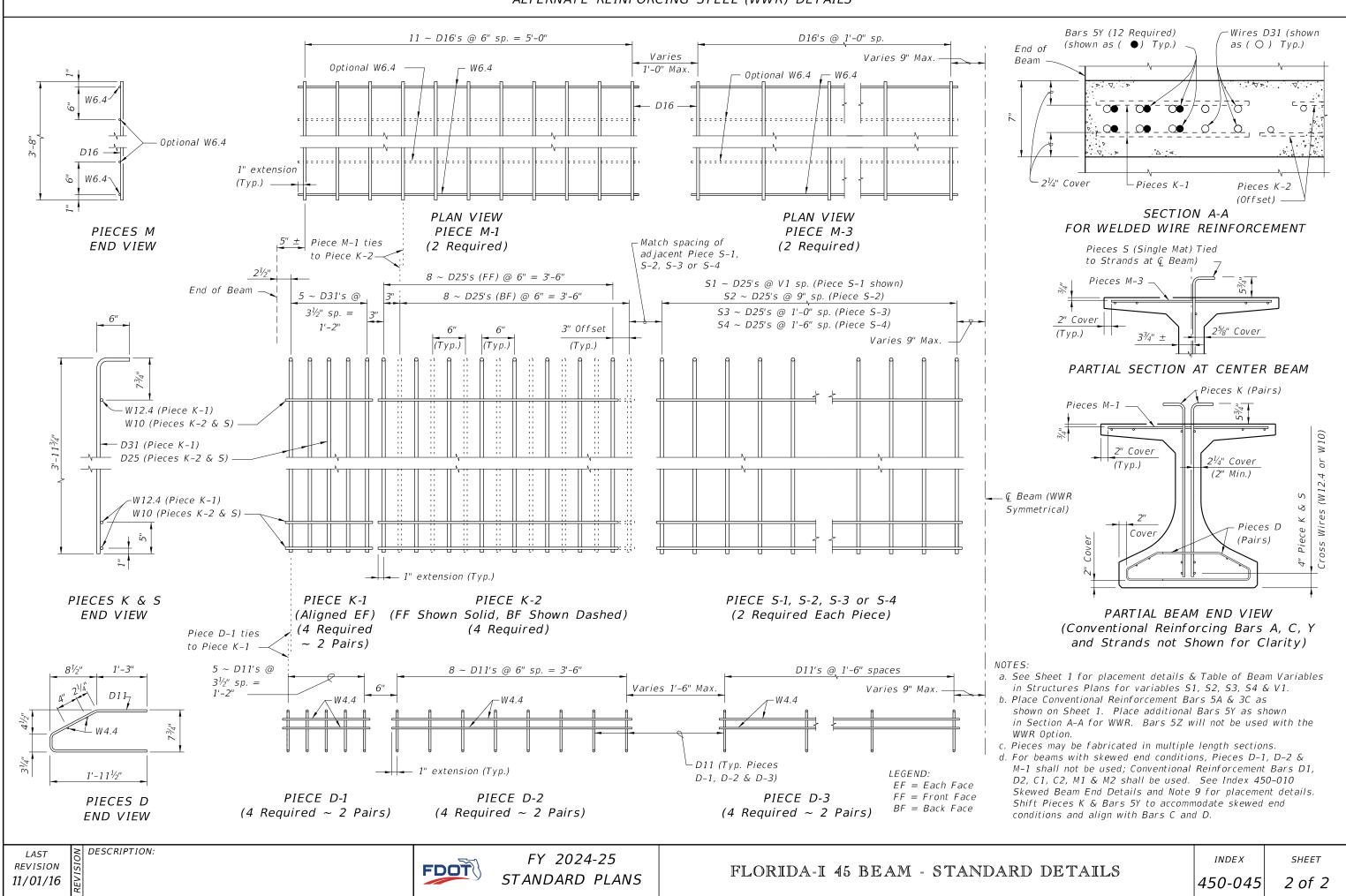
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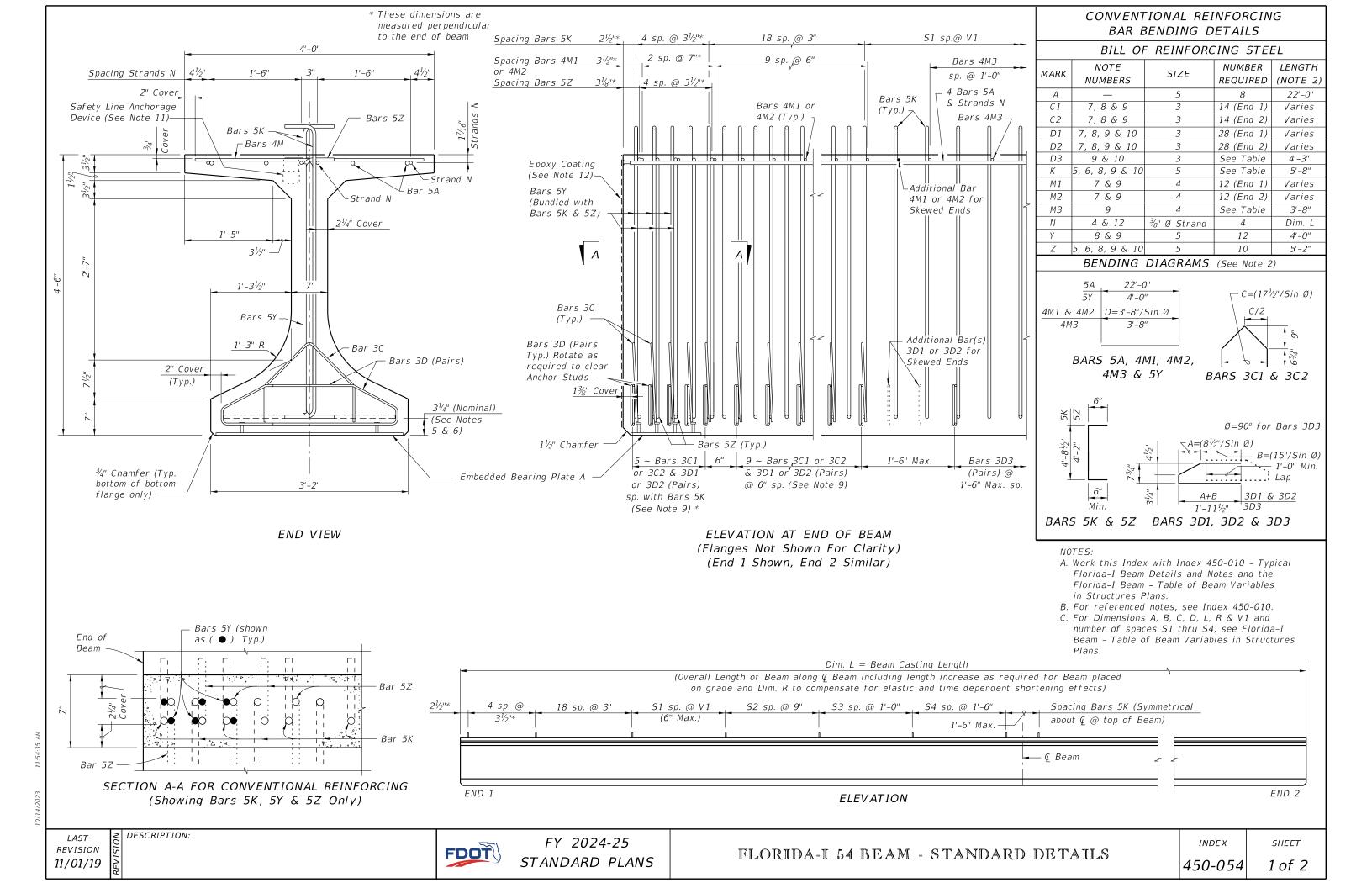
2 of 2 450-010

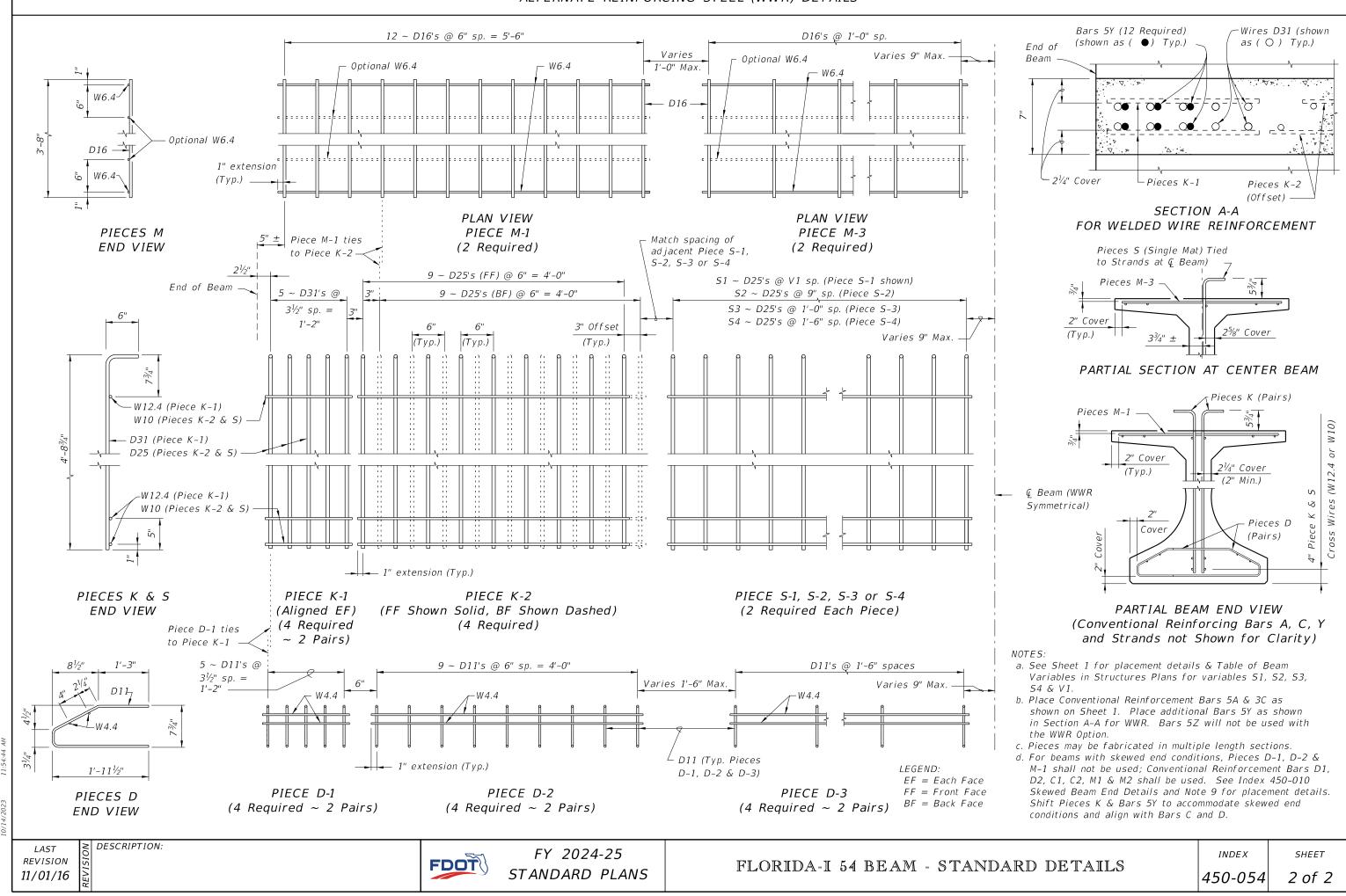


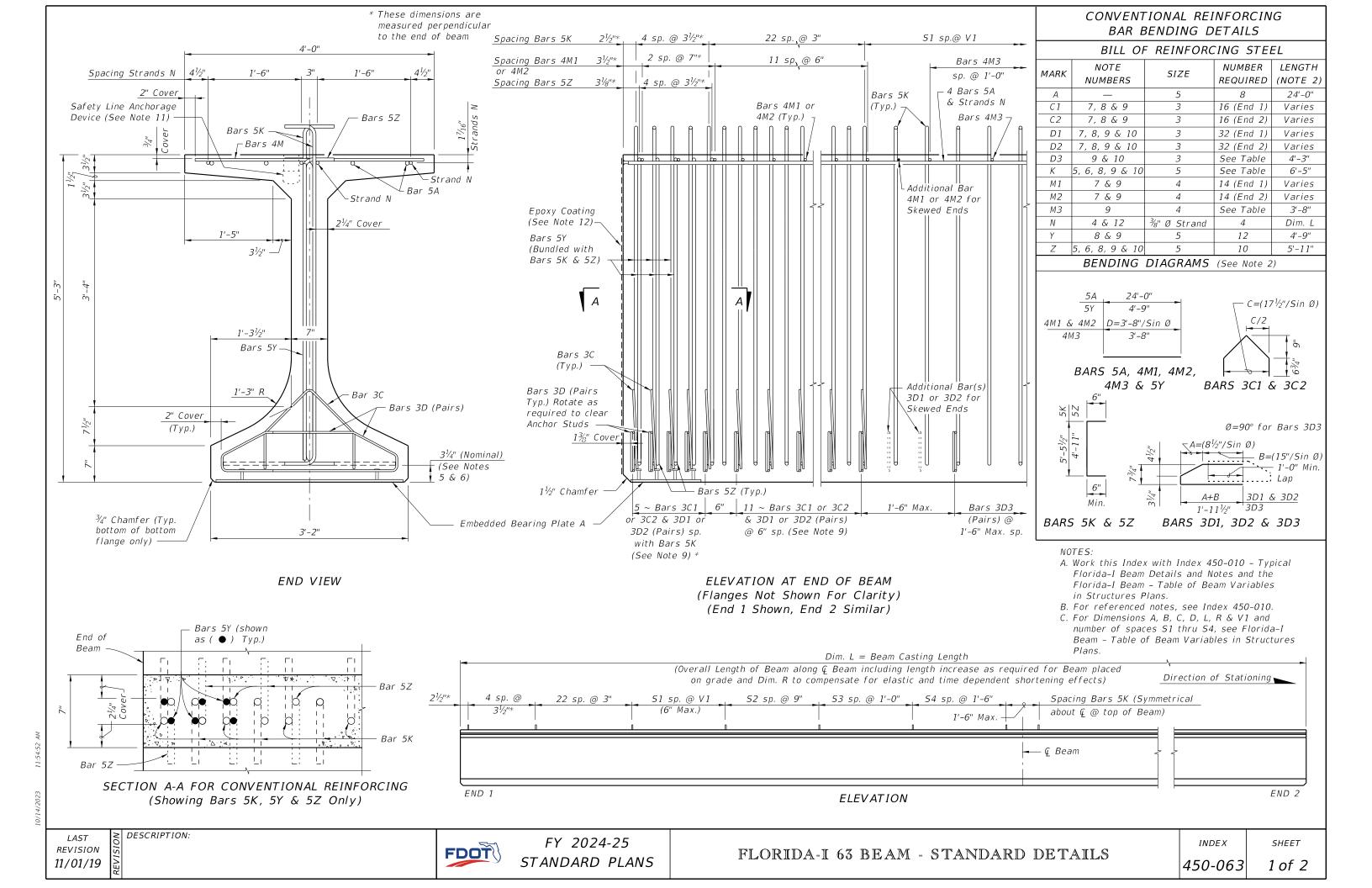


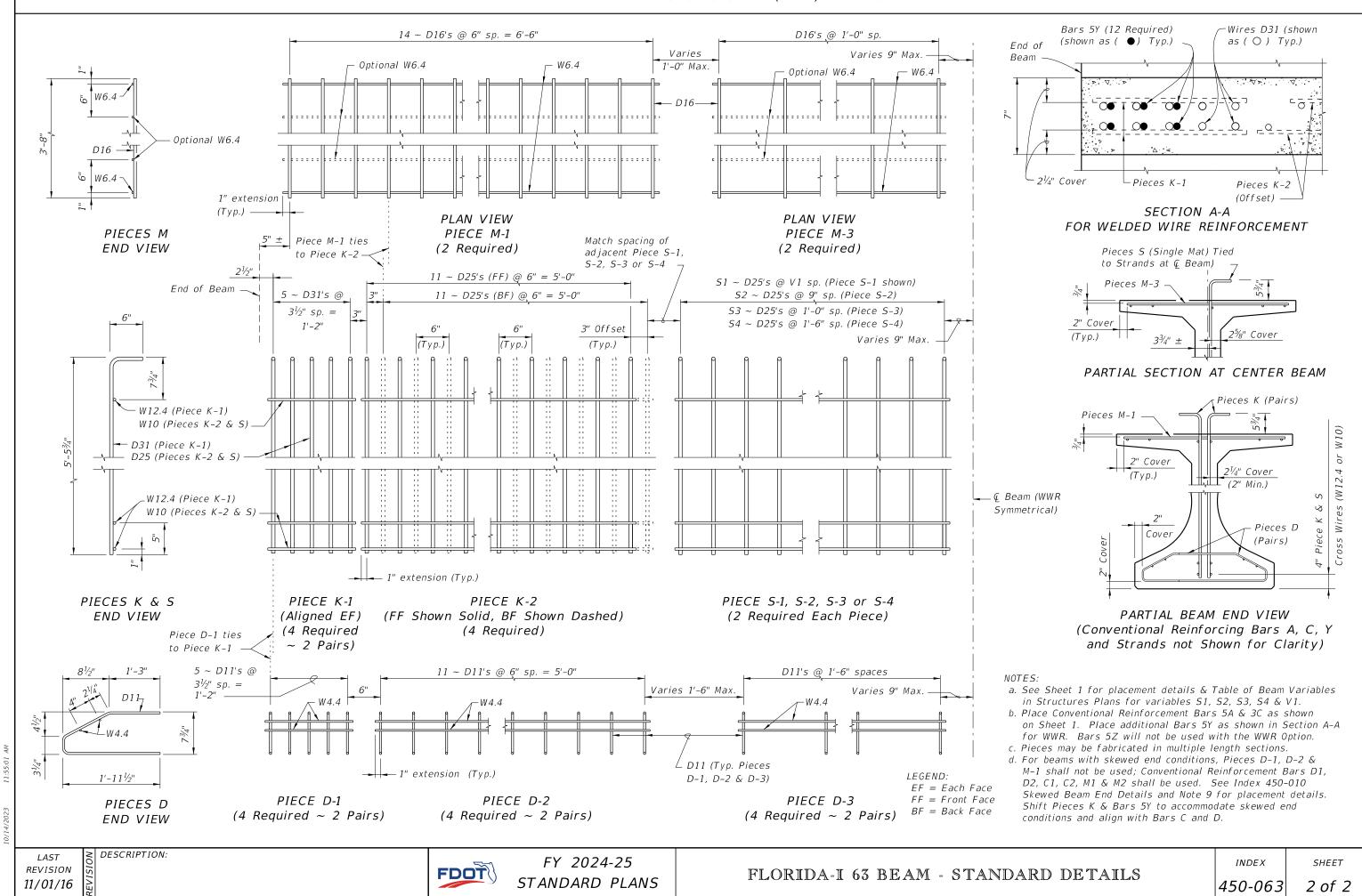


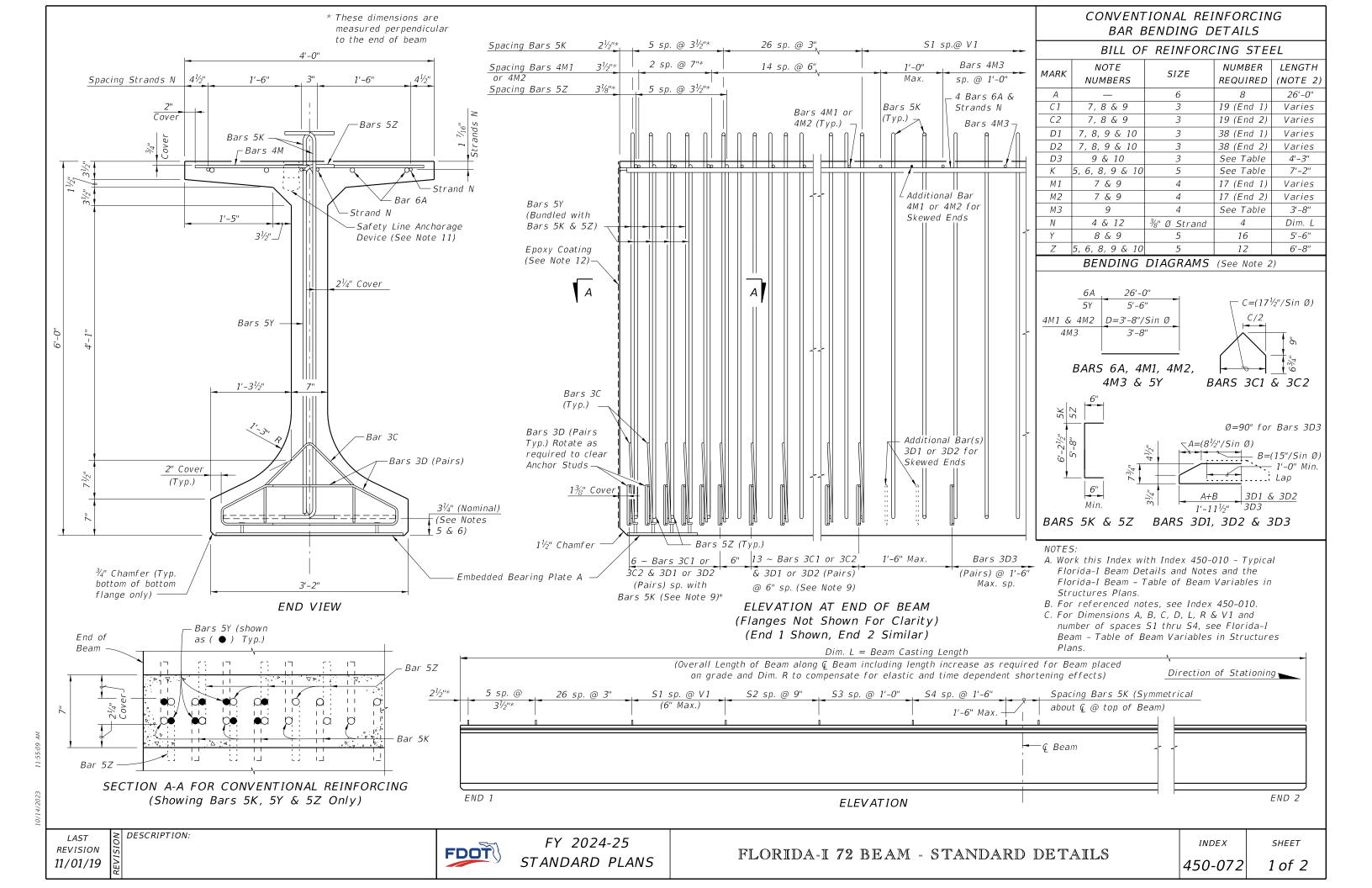


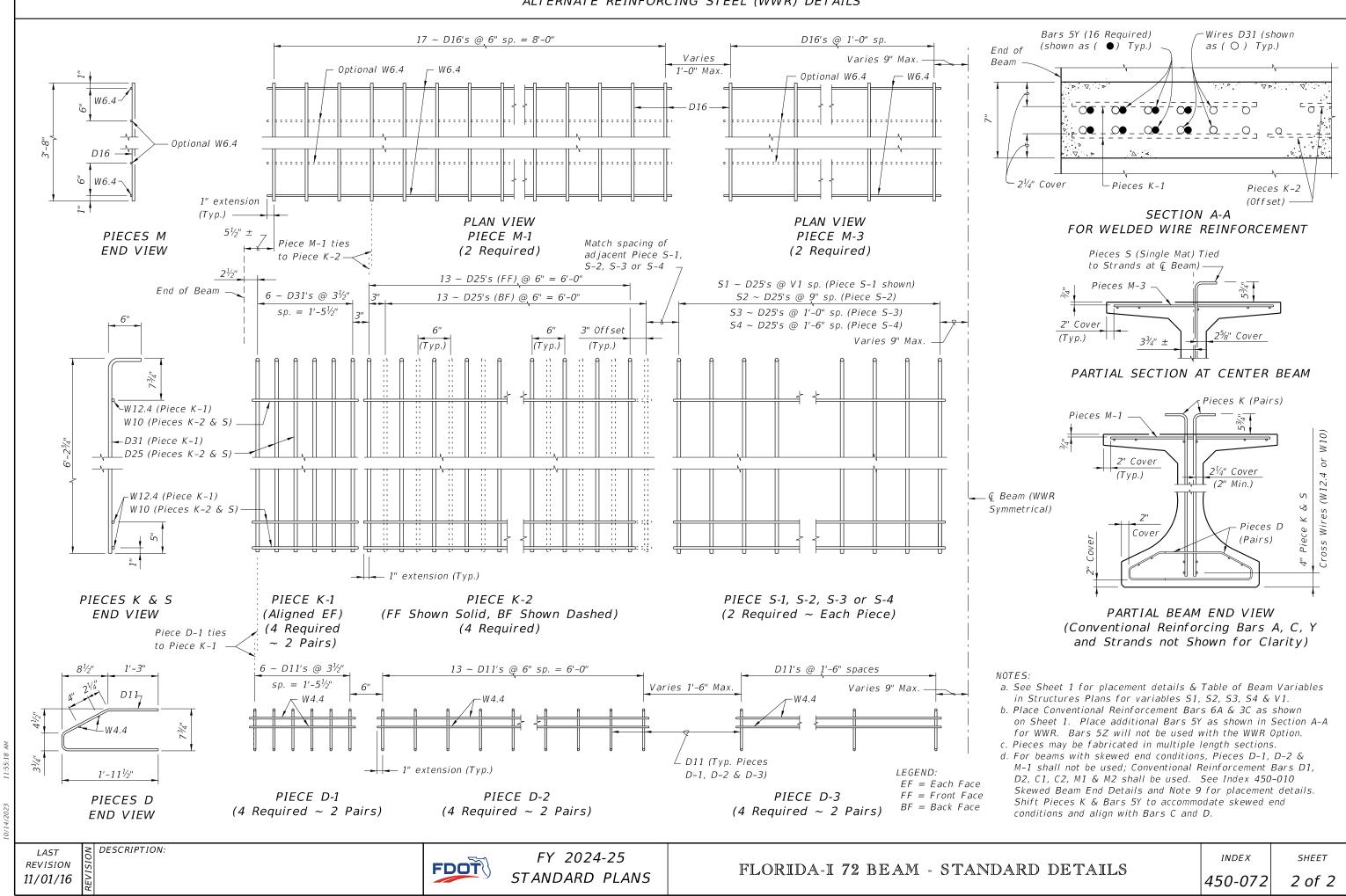




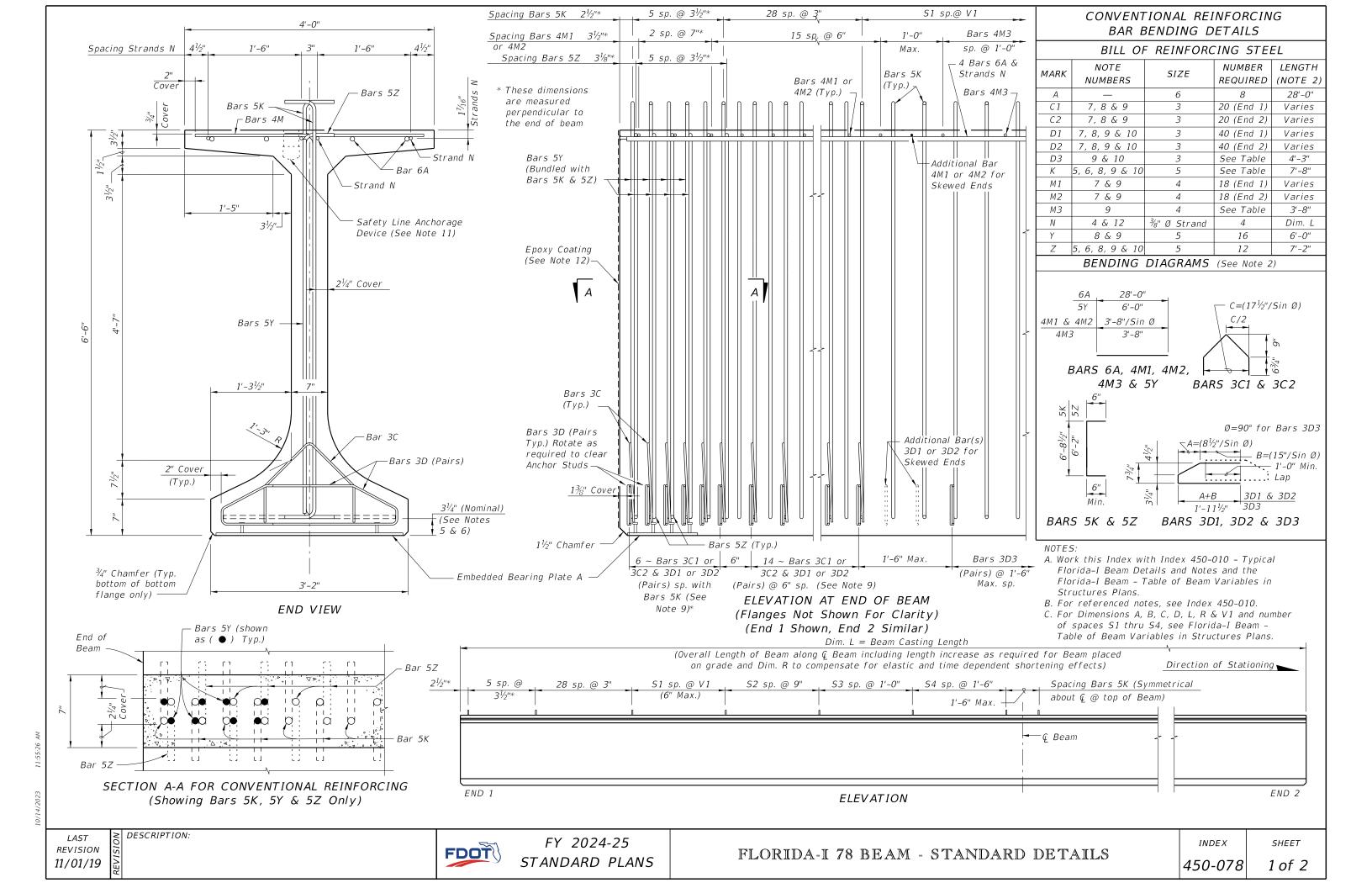


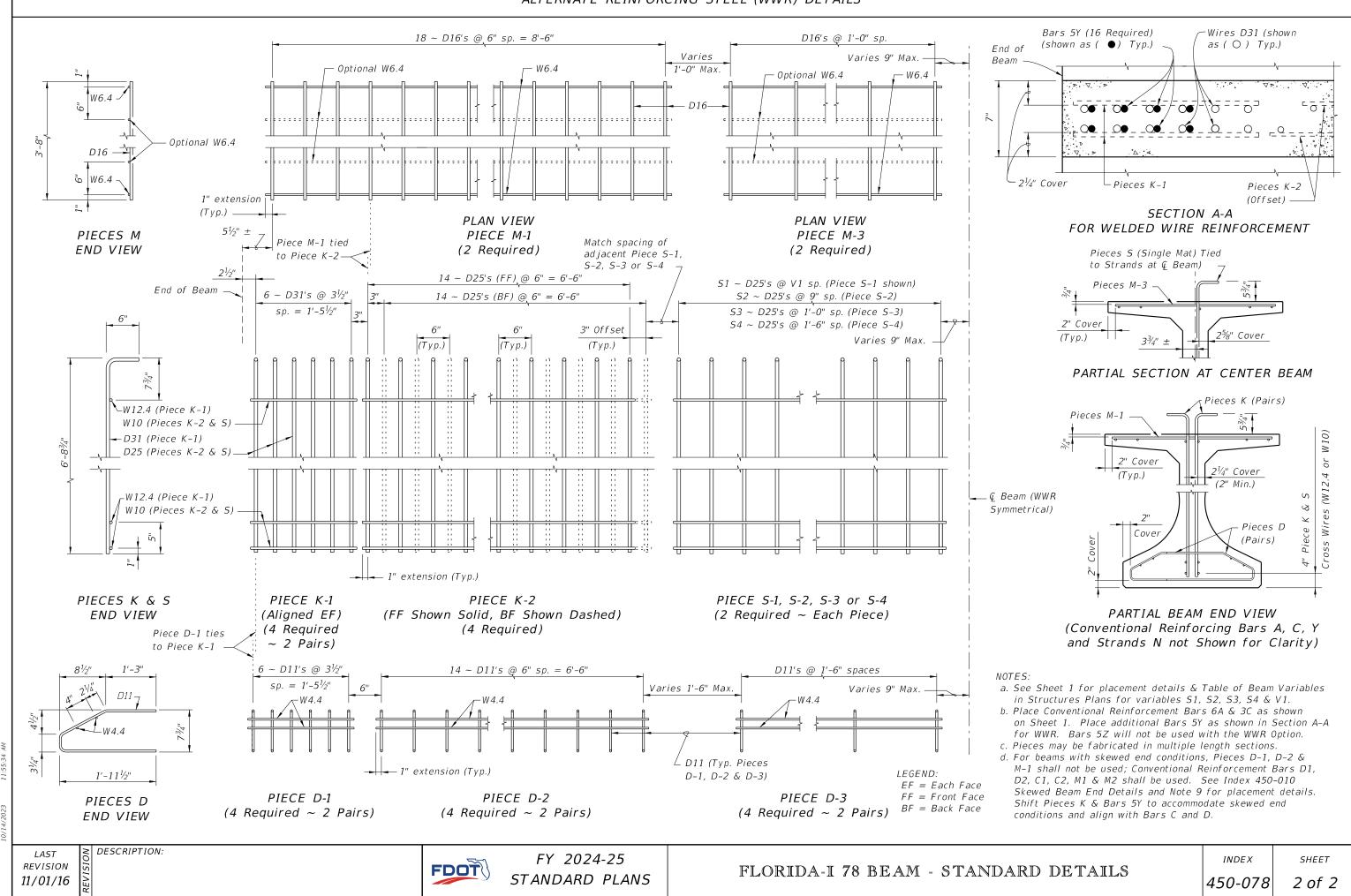


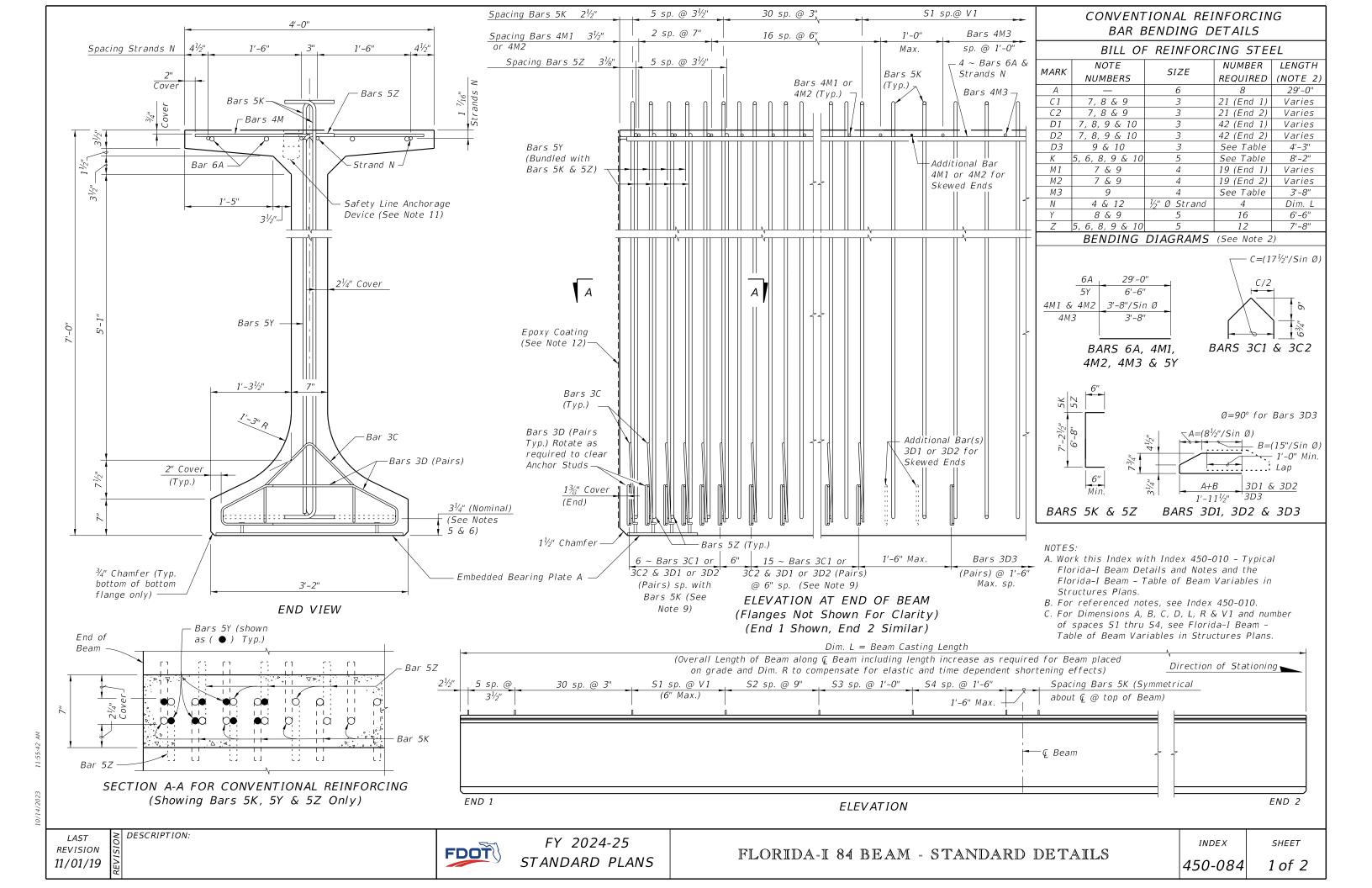


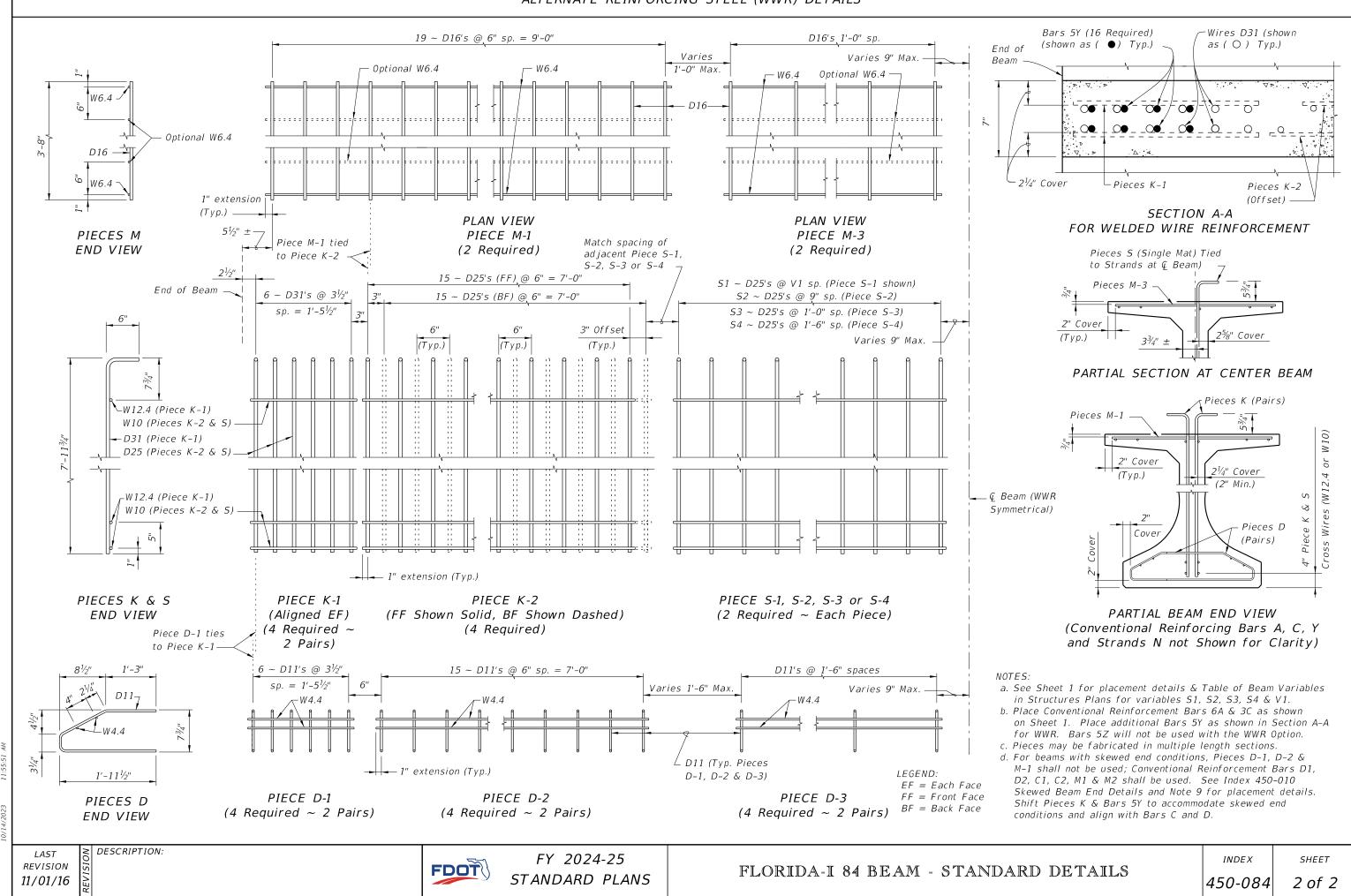


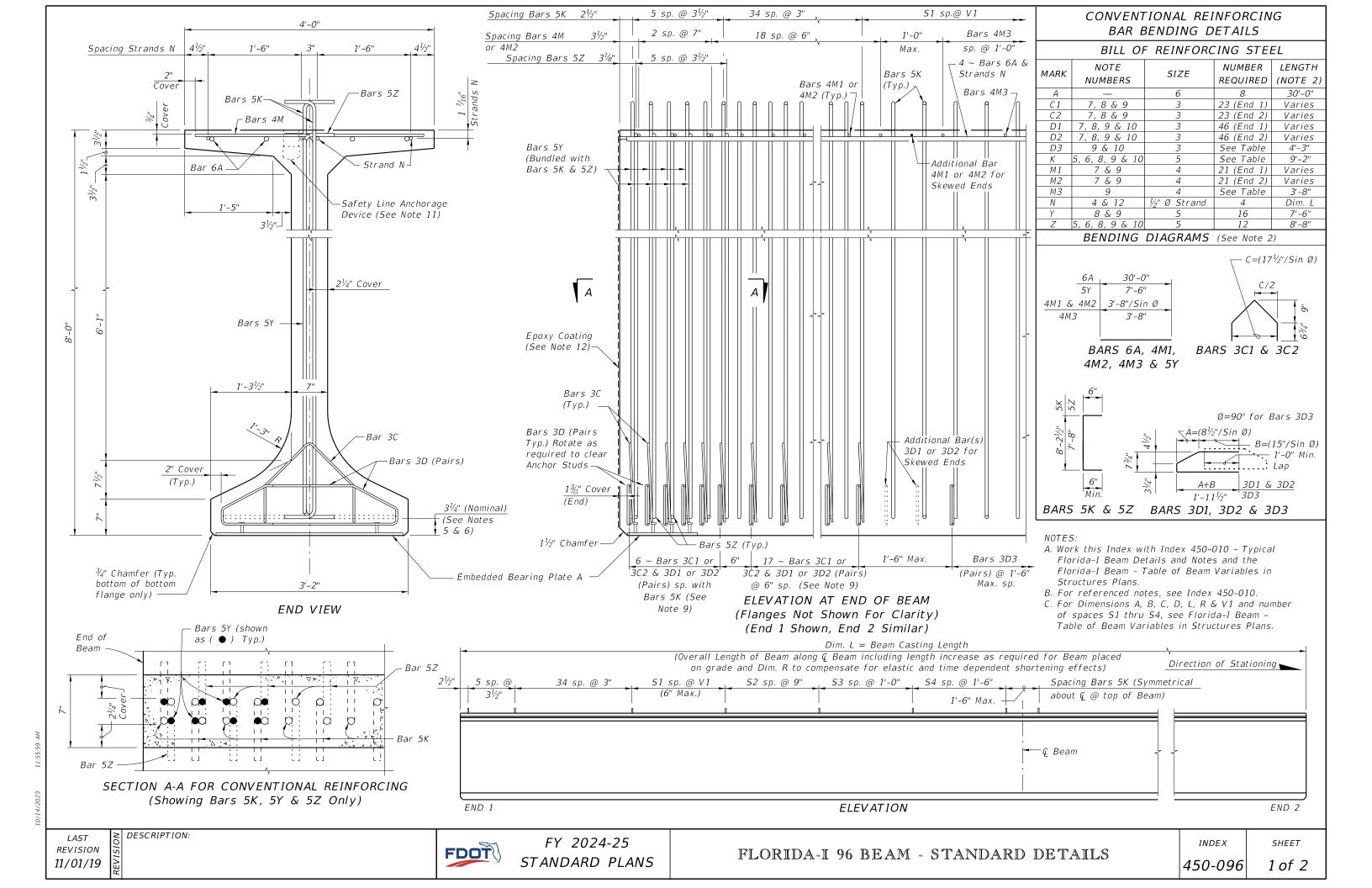
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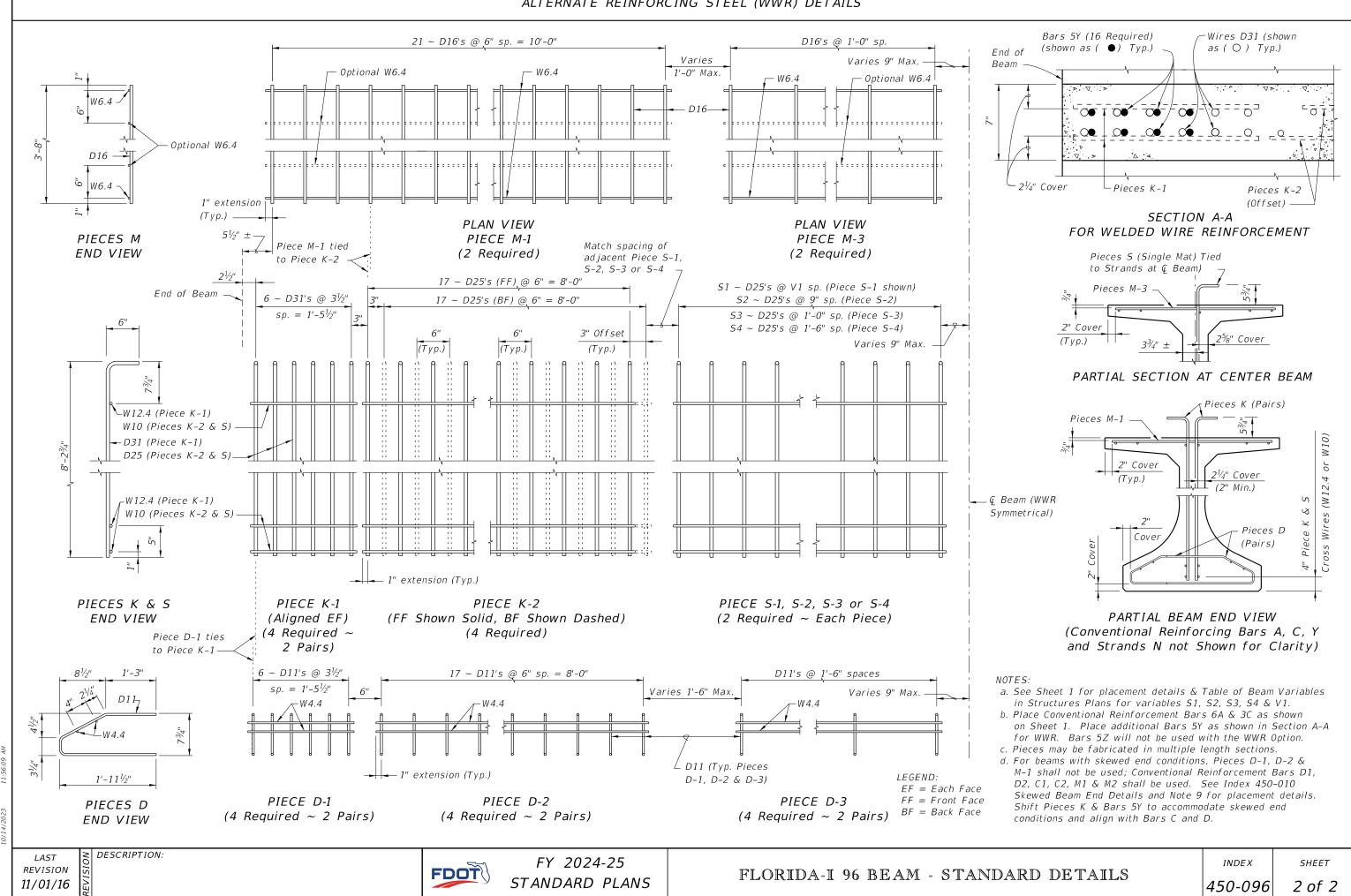


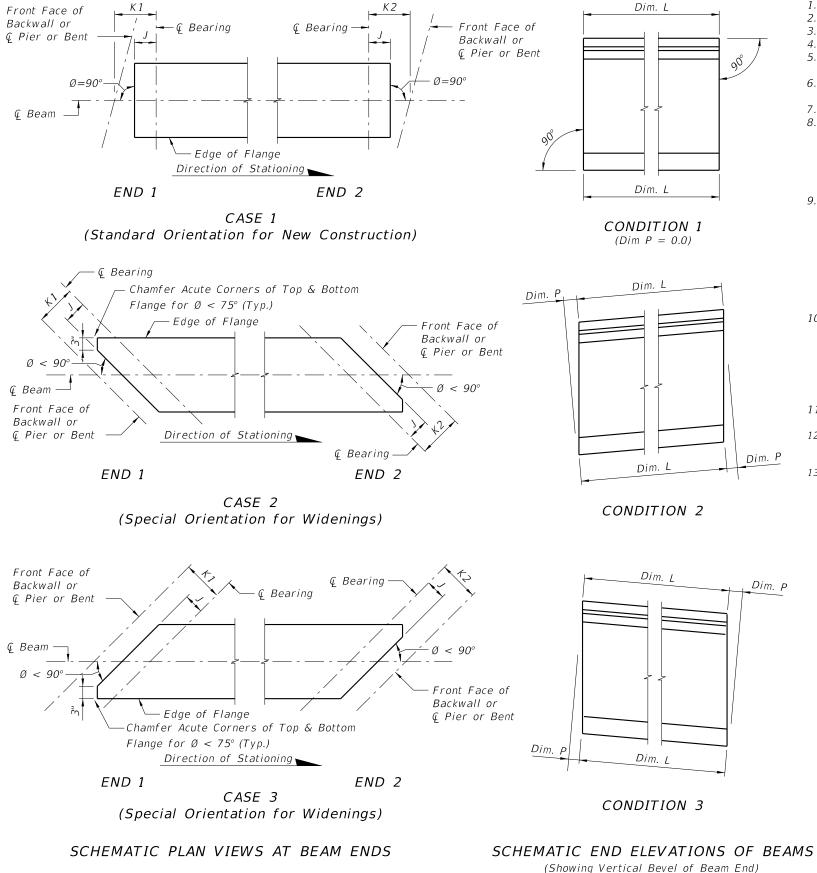












- Work this Index with the Table of Beam Variables in Structures Plans.
- All bar bend dimensions are out to out.
- 3. Concrete cover: 2 inches minimum.
- 4. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 5. Place one (1) Bar 4K or 5Z at each location. Alternate the direction of the ends for each
- 6. Tie Bars 4K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. Place Bars 3D1 in beam END 1, and Bars 3D2 in beam END 2.
- 8. For Beams with vertically beveled end conditions:
 - A. Place first row of Bars 3D1, 3D2, 4K, 4Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1".
 - For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
- 9. For beams with skewed end conditions:
 - WWR is not permitted for end reinforcement Bars 3D1, and 3D2 on skewed ends; use bar reinforcement.
 - Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3D1, 3D2, 4K, 4Y and 5Z placed within the limits of the spacing for Bars 3D in "ELEVATION AT END OF BEAM"
 - Beyond the limits of the spacing for Bars 3D, place Bars 4K perpendicular to the longitudinal axis of the beam. For placement see "SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES" (Sheet 2).
- 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 4K, and 5Z as shown on Sheet 4; except at skewed ends (See Note 9).
 - Bars 3D1 and 3D2 may be fabricated as a two-piece bar with a 1'-0" minimum lap splice of the bottom legs.
- For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands or above Strands N.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of required anchorage devices.
- 12. For beams with ends that will not to be encased in concrete diaphragms, cut wedges and recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
- The superstructure environmental classification is slightly or moderately aggressive
- Clear cover to adjacent steel reinforcing is 1"or greater
- Hole inside diameter is 2" maximum
- Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

DETAILS AND NOTES

LAST REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2024-25 STANDARD PLANS

AASHTO TYPE II BEAM

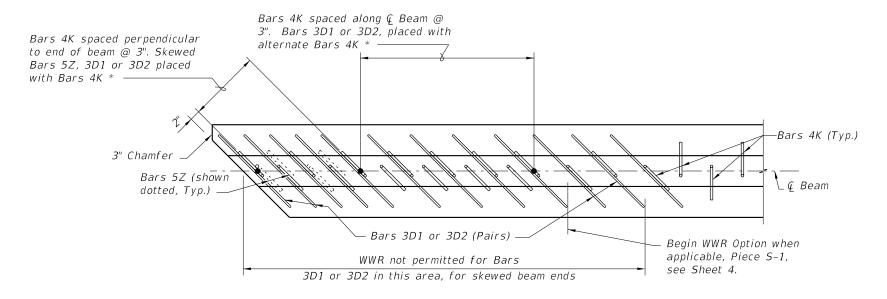
INDEX

SHEET

450-120 1 of 4

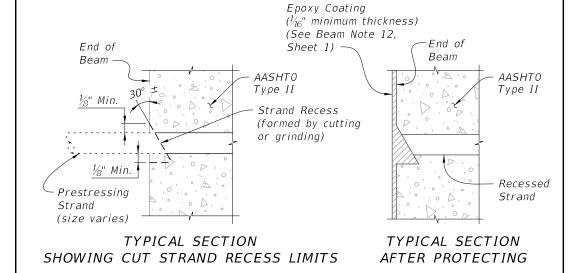
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for WWR.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE) (End 1 Shown, End 2 Similar) (Bars 4Y & Strands not shown for clarity)

SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

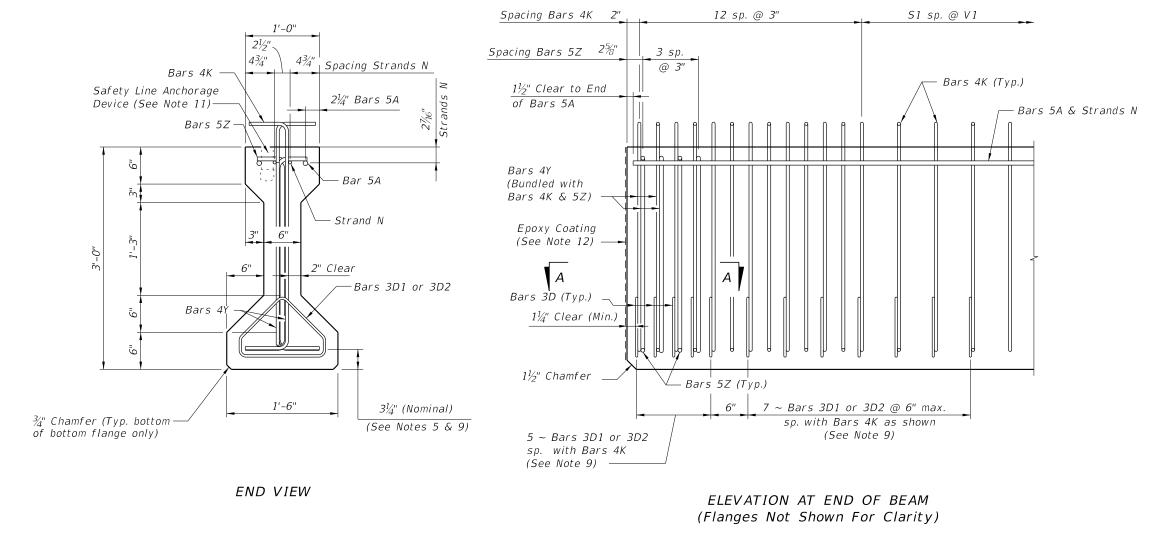
REVISION 11/01/19

FY 2024-25 STANDARD PLANS INDEX

SHEET 450-120 2 of 4

DESCRIPTION:

FDOT



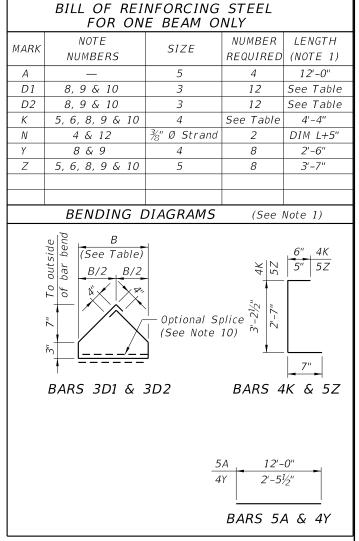
(Showing Bars 4K, 4Y & 5Z Only)

DESCRIPTION:

LAST

REVISION

11/01/19



NOTES

AASHTO TYPE II BEAM

Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.

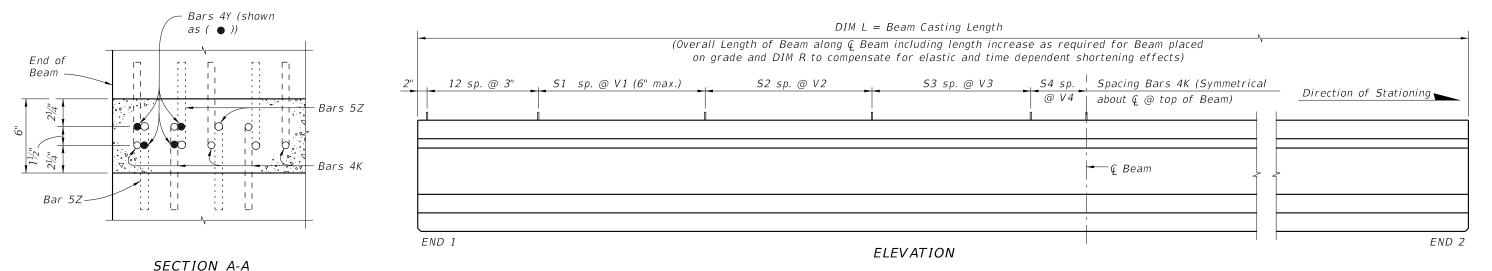
STANDARD DETAILS

SHEET

3 of 4

INDEX

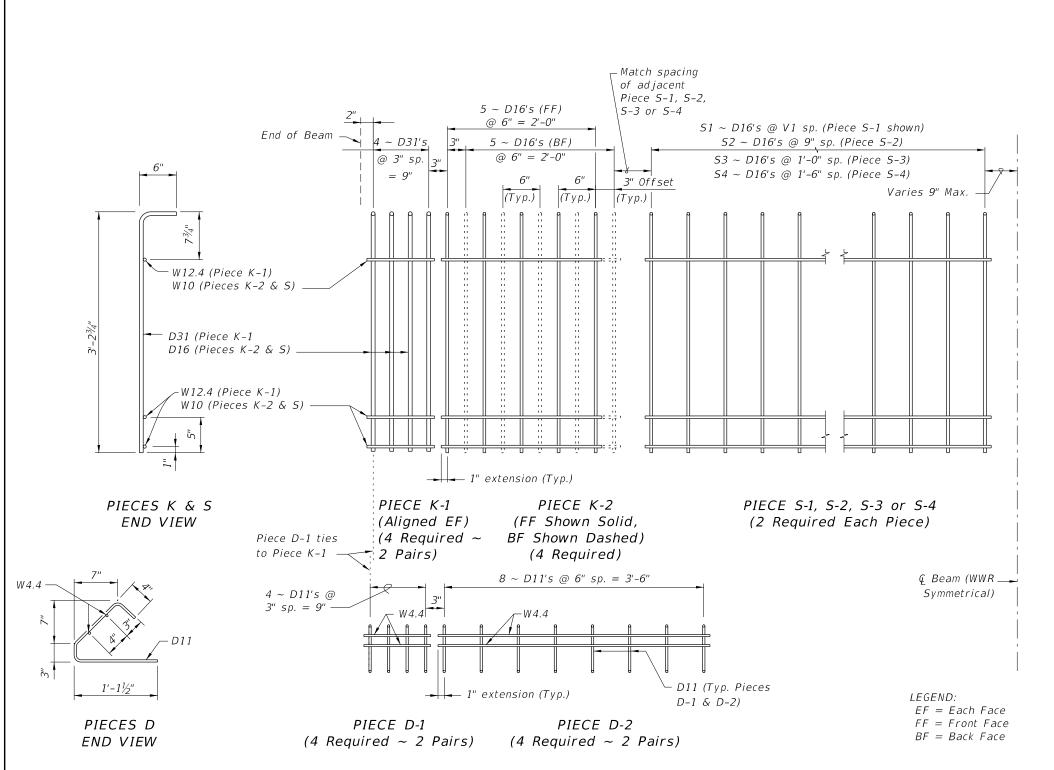
450-120

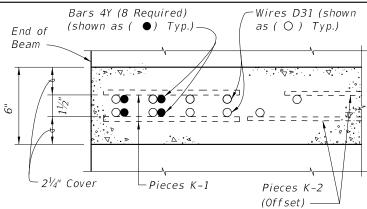


FY 2024-25

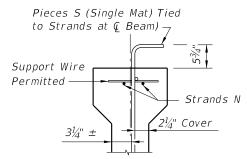
STANDARD PLANS

FDOT

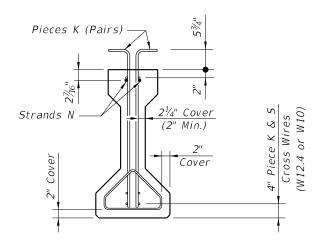




SECTION A-A FOR WELDED WIRE REINFORCEMENT



PARTIAL SECTION AT CENTER BEAM



PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, 54 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

STANDARD DETAILS

LAST REVISION 11/01/16

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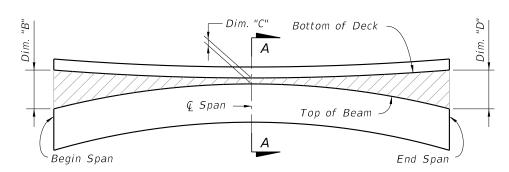
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FY 2024-25 STANDARD PLANS

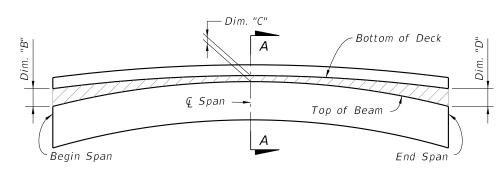
INDEX SHEET

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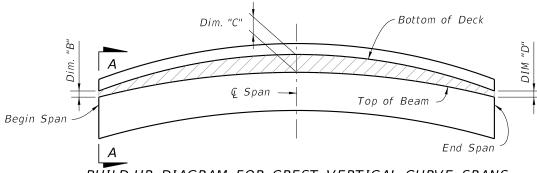
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG G BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT G SPAN (ALONG Q BEAM) (CASE 3)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG Q BEAM) (CASE 4)

DESCRIPTION: REVISION 11/01/21

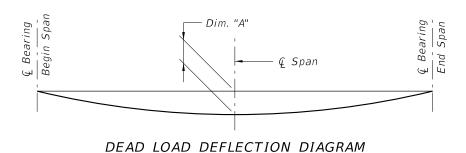


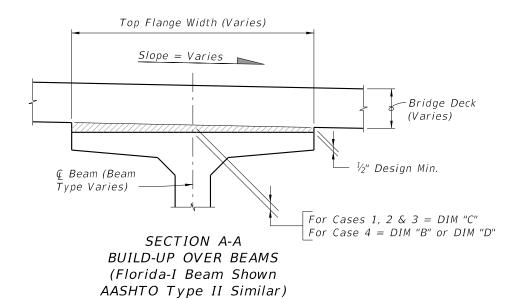
FY 2024-25 STANDARD PLANS

BEAM CAMBER AND BUILD-UP NOTES:

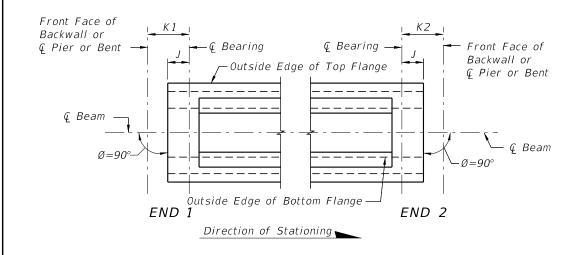
The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.

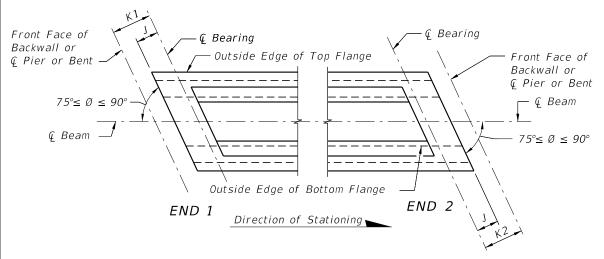




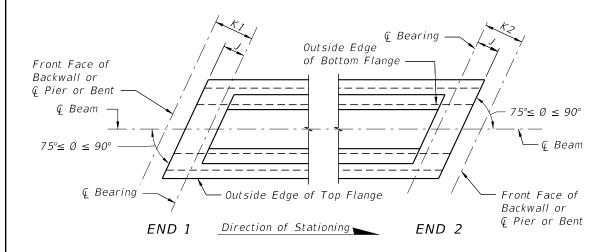
* NOTE: Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.



CASE 1



CASE 2



CASE 3

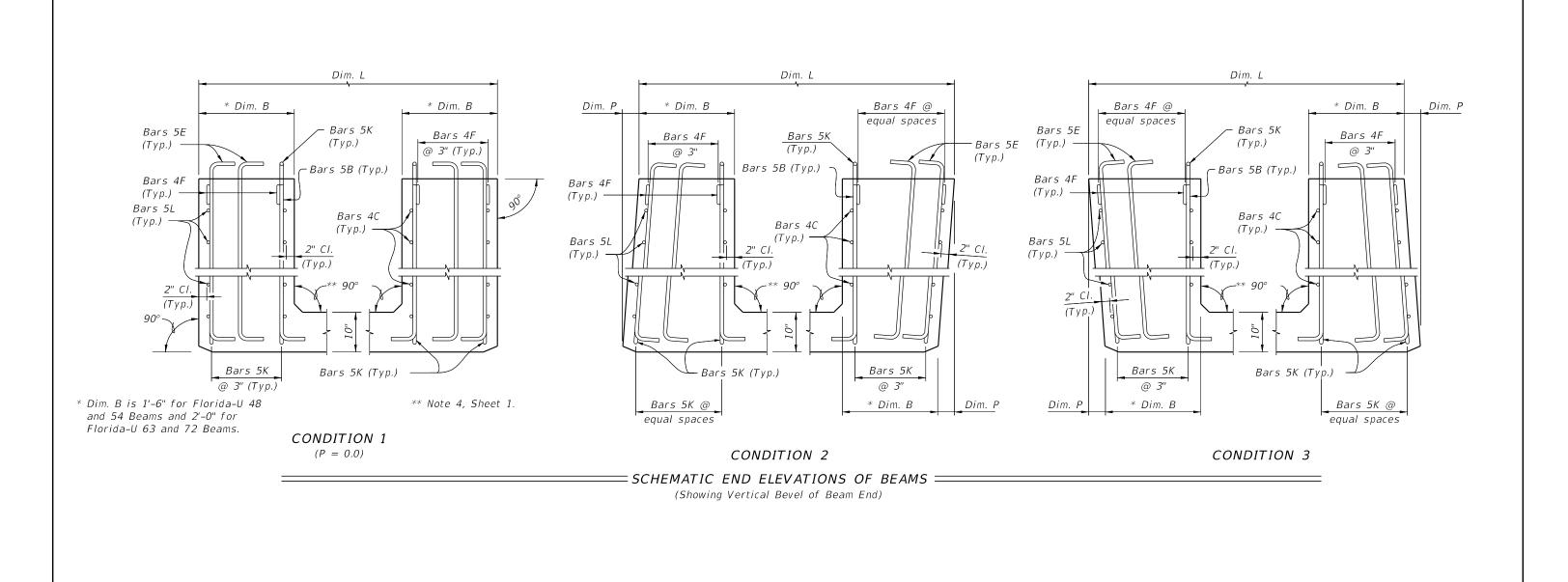
= SCHEMATIC PLAN VIEWS AT BEAM ENDS =

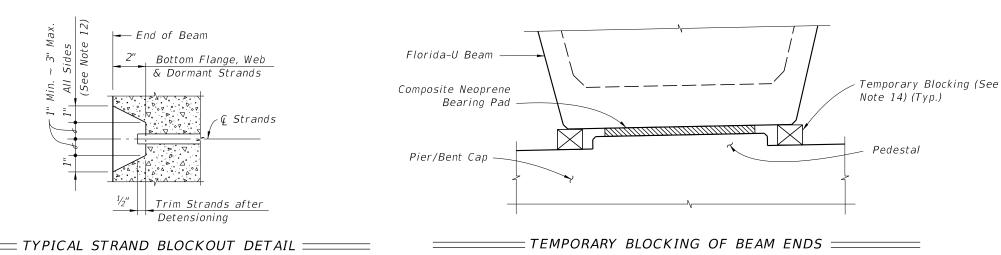
BEAM NOTES

- 1. Work this Index with the Florida-U Beam Standard Details (Index 450-248, 450-254, 450-263 and 450-272) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum. Maximum aggregate size is a No. 67.
- 4. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 5. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 6. Tie Bars 5K to the fully bonded strands in the bottom row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. For beams without skewed ends or vertically beveled end conditions (see Note 8) the Engineer may approve the use of deformed WWR in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M. The spacing and sizes of deformed WWR must match the reinforcing sizes shown on the Florida-U Beam Standard Details sheets.
- 8. For Beams with vertically beveled end conditions, where "Dim. P" exceeds 1", place Bars 5E, and the first Bars 4F and 5K parallel to the end of the beam. Fan the remaining Bars 4F and 5K within the limits of "Dim. B" (End Diaphragm) at equal spaces until vertical.
- 9. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any anchorage devices or other required embedded hardware.
- 10. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing the beam from casting bed.
- 11. Place drains pipes adjacent to each web at each beam end (four drains per beam).

 A. Drain Pipe: 2" NPS Schedule 80 PVC.
 - A. Drain Pipe: 2" NPS Schedule 80 PVC.B. Cover, wrap and secure wire screen around the end of the pipe prior to casting.
 - B. Cover, wrap and secure wire screen around the end of the pipe prior to casting. Extend screen a minimum of 1" down the pipe sides.
 - C. Provide removable pipe plugs during casting. Remove plugs from the inside of pipes after casting.
- 12. Protection of Strands:
 - A. Provide a 2" deep recess around all strands (including dormant) or strand groups. Extend the recessed blockout to the web face and bottom of the flange for the bottom row of strands.
 - B. After detensioning, cut strands ½" from recessed surface and fill the blockout to protect strands with Type F-2 or Q Epoxy Compound in accordance with Specification Section 926.
- 13. Use Stay-In-Place metal deck forms inside the beams.
- 14. Prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations during deck placement. Leave temporary blocking and bracing in place for a minimum of four days after the deck is placed.
- 15. Based on the deck forming system and deck placement sequence, evaluate and provide any required temporary bracing between the U Beams.

DESCRIPTION:



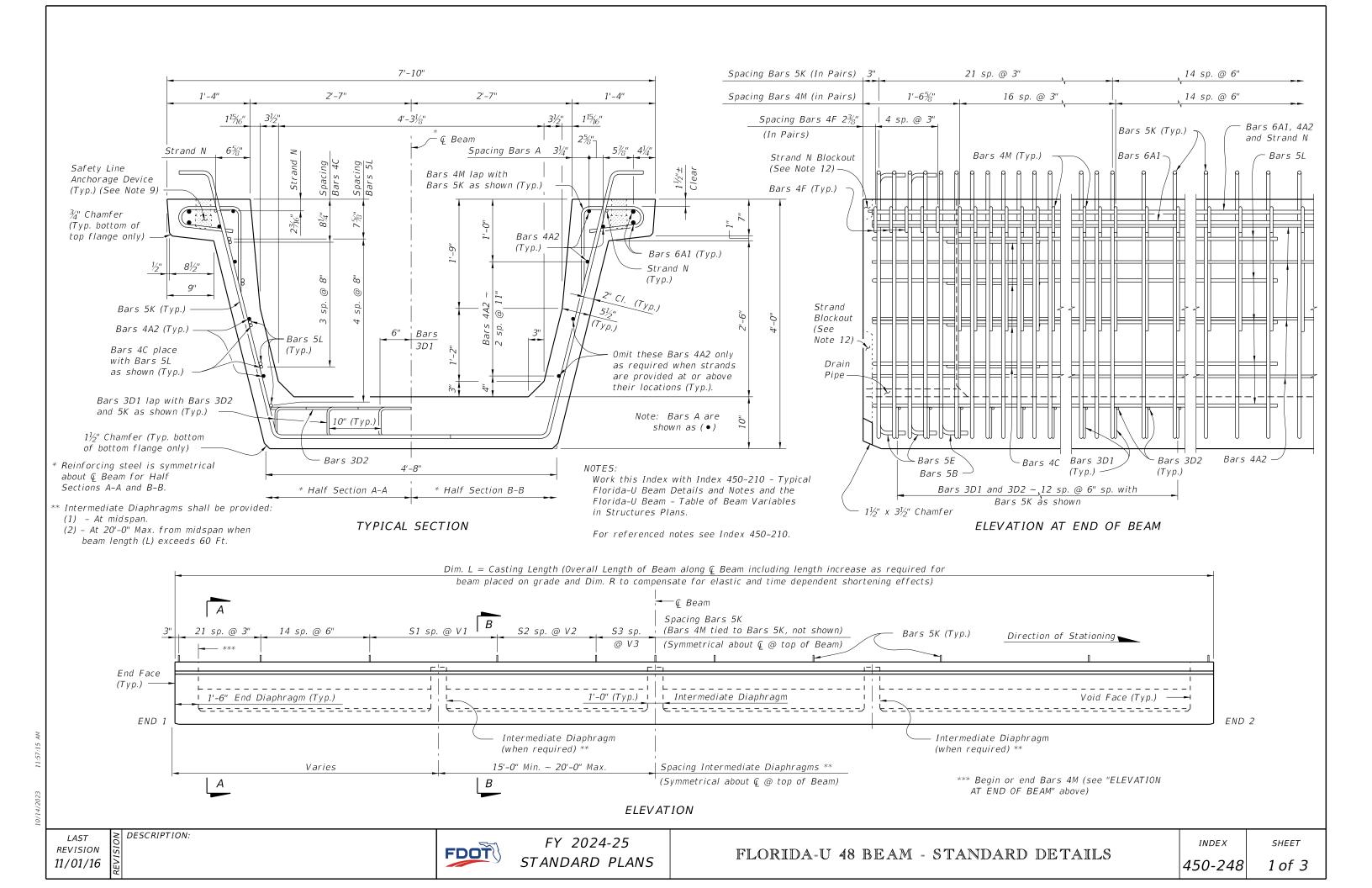


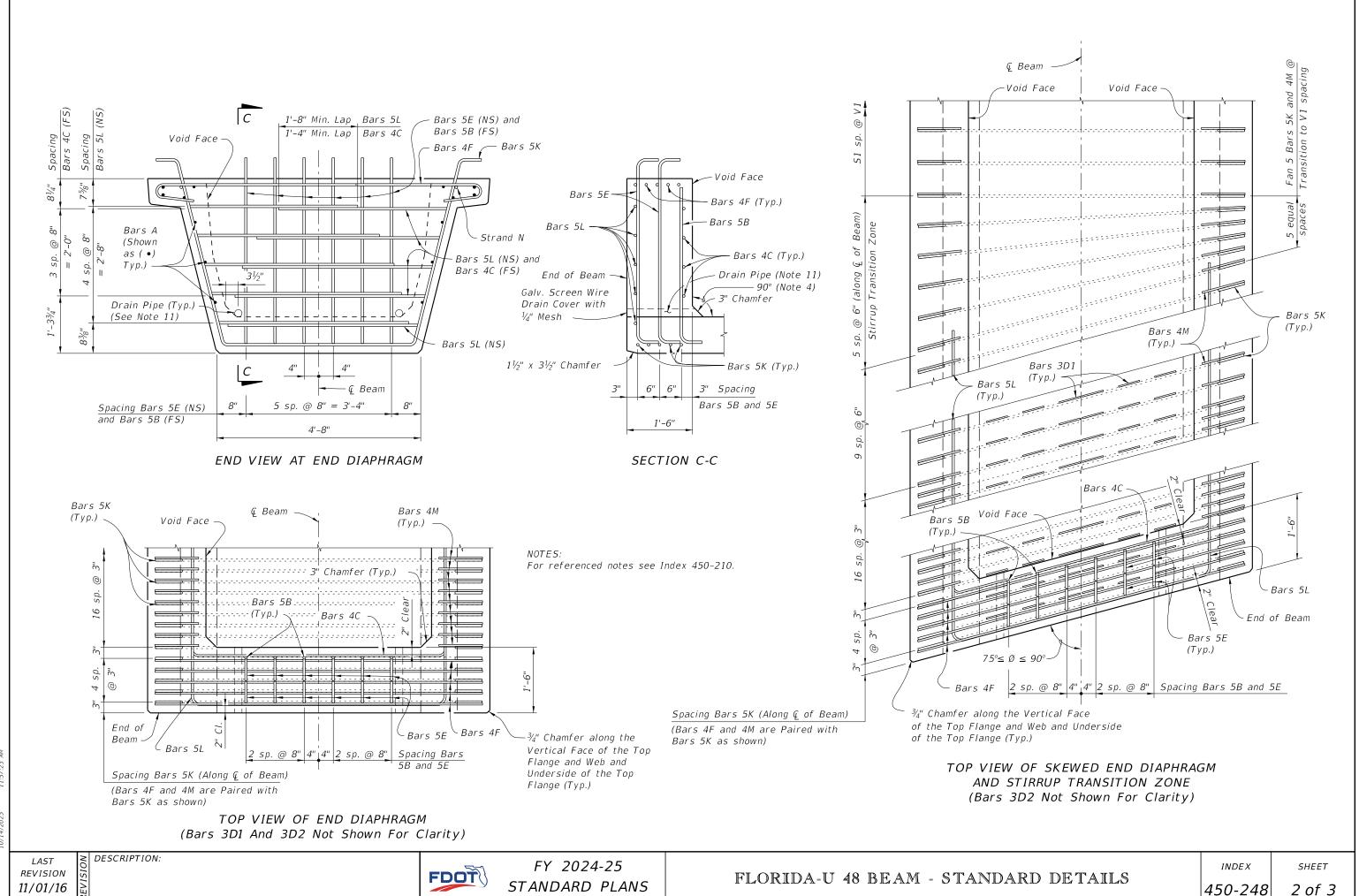
REVISION 11/01/16

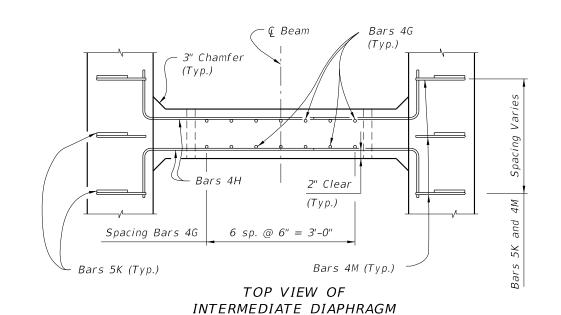
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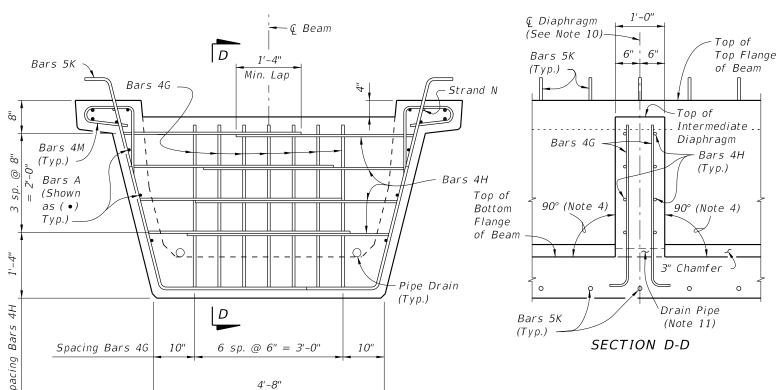
FDOT

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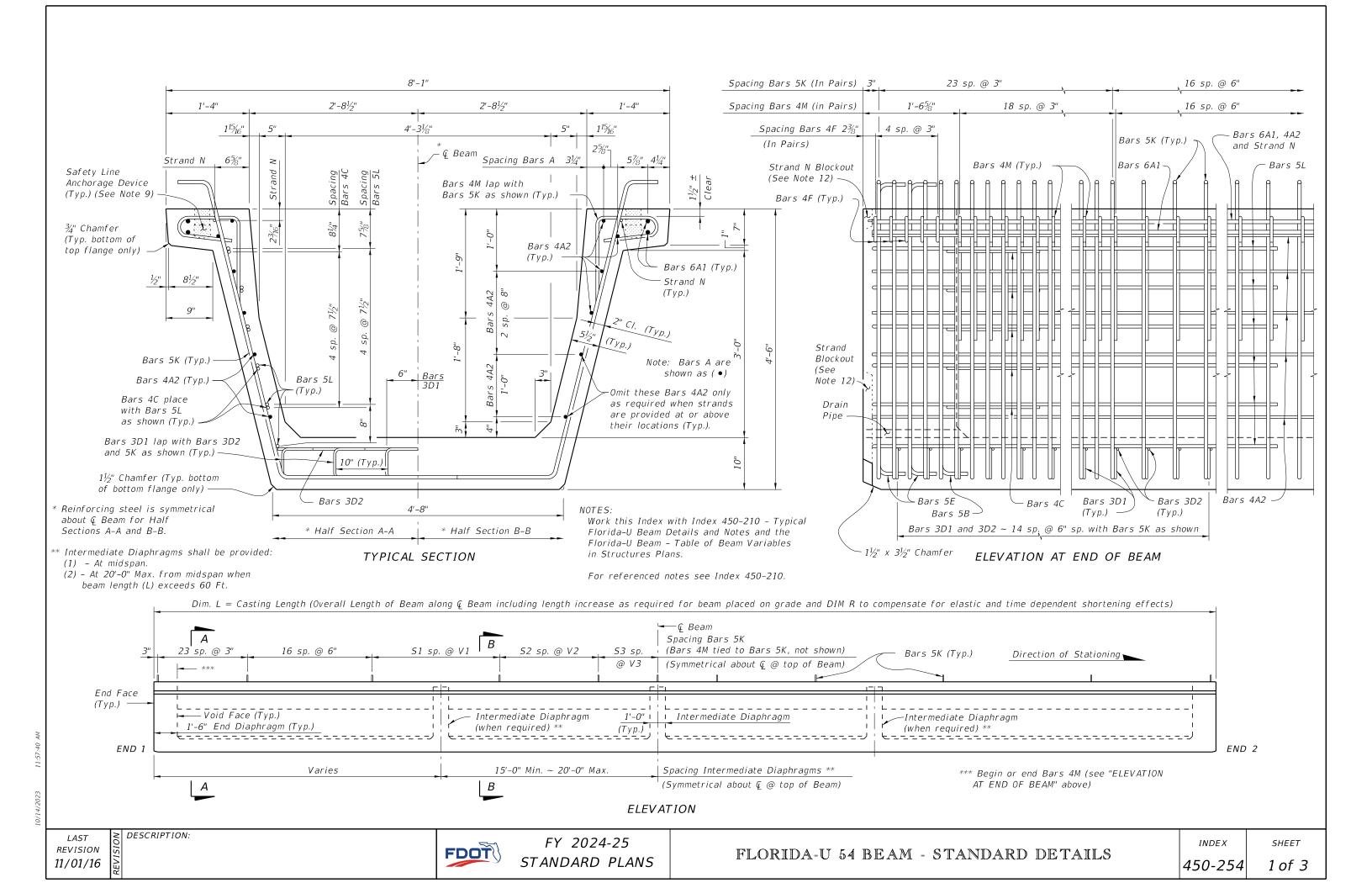


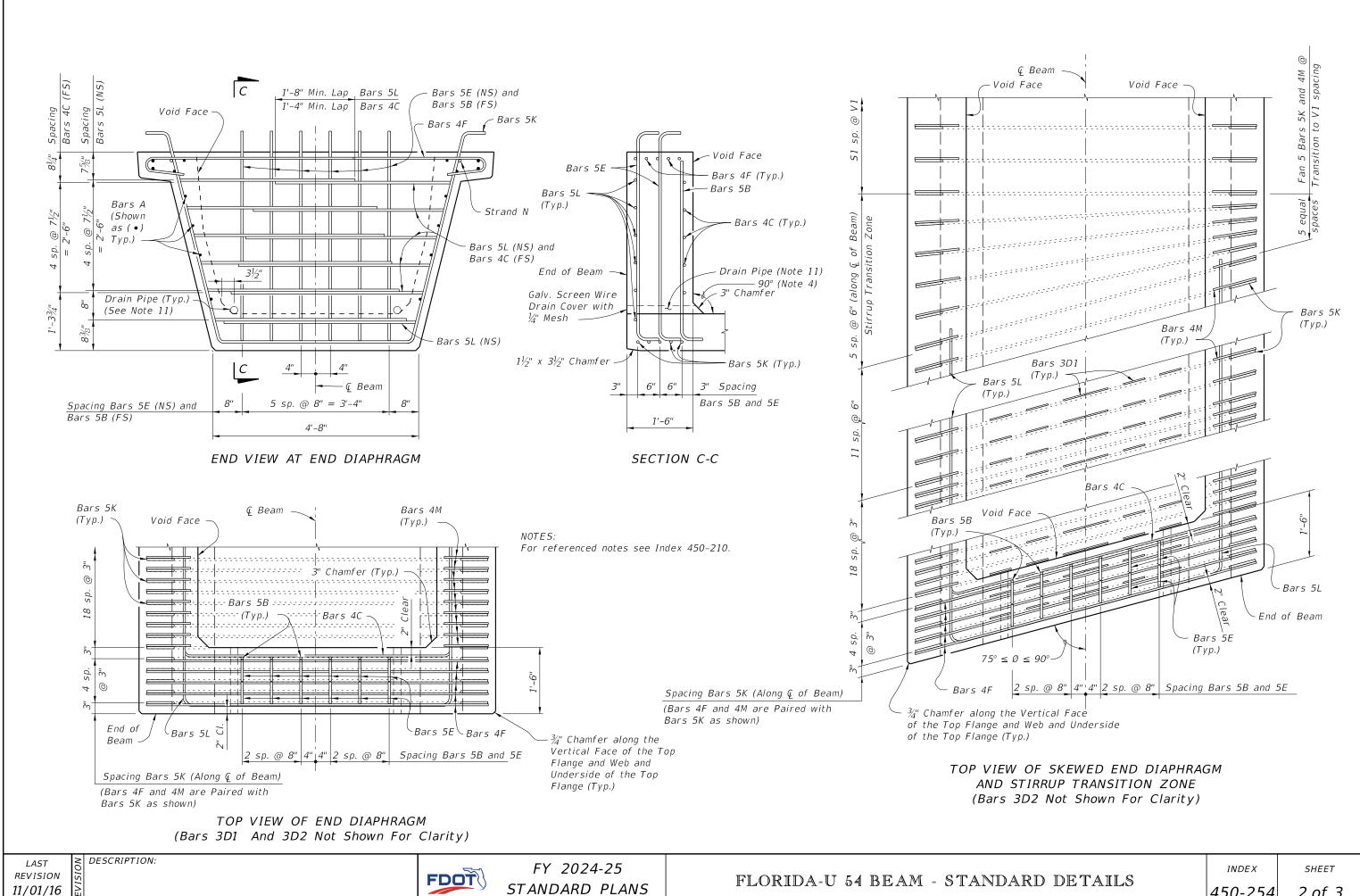


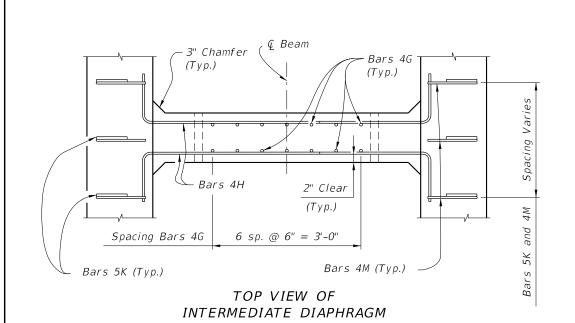
SECTION AT INTERMEDIATE DIAPHRAGM

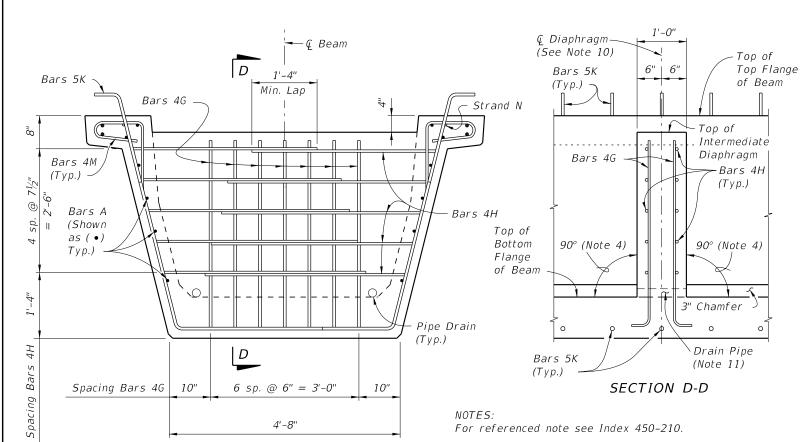
NOTES: For referenced notes see Index 450-210.

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL FOR ONE BEAM ONLY MARKSIZE NO. REQD. LENGTH A1 6 Dim. L - 4" 4 A2 10 Dim. L - 4" В 5 12 4'-1" 4 16 5'-1" D1 3 156 1'-6" Bars 3D1 D2 3 26 4'-6" Ε 5 24 5'-3" Bars 5B 4 20 F 6'-2" 6" G 4 See Table 4'-0" L - 4" (Min. Lap Splice = 2'-0") Bars 5E 4A2 L - 4" (Min. Lap Splice = 1'-4") 4 4'-7" Н See Table 3D2 4'-6" 5 See Table 8'-0" 5 20 14'-0" Μ 4 See Table 3'-11" Bars 6A1, 4A2 and 3D2 Ν ¾" Ø Strand 2 Dim. L - 3" Field Bend as Required for Skew 5'-2" 3" Ø Pin 3'-7" Bars 4C Bars 4F 3'-7" 8" Bars 4G Bars 4H 1'-0" 1'-0" 3" Ø Pin--Field Bend as Required for Skew 4'-0" 1'-07/8" 3'-0" Bars 5L Bars 4M Bars 5K







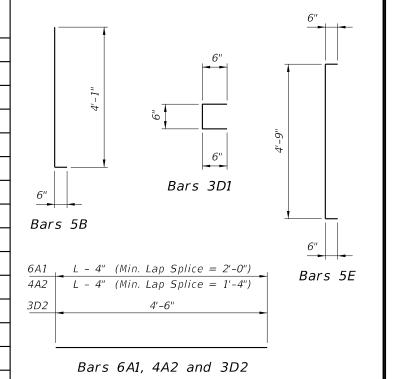


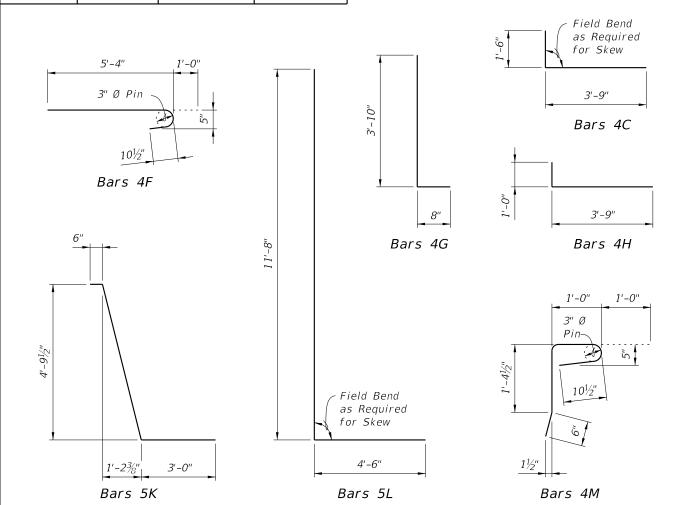
SECTION AT INTERMEDIATE DIAPHRAGM

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

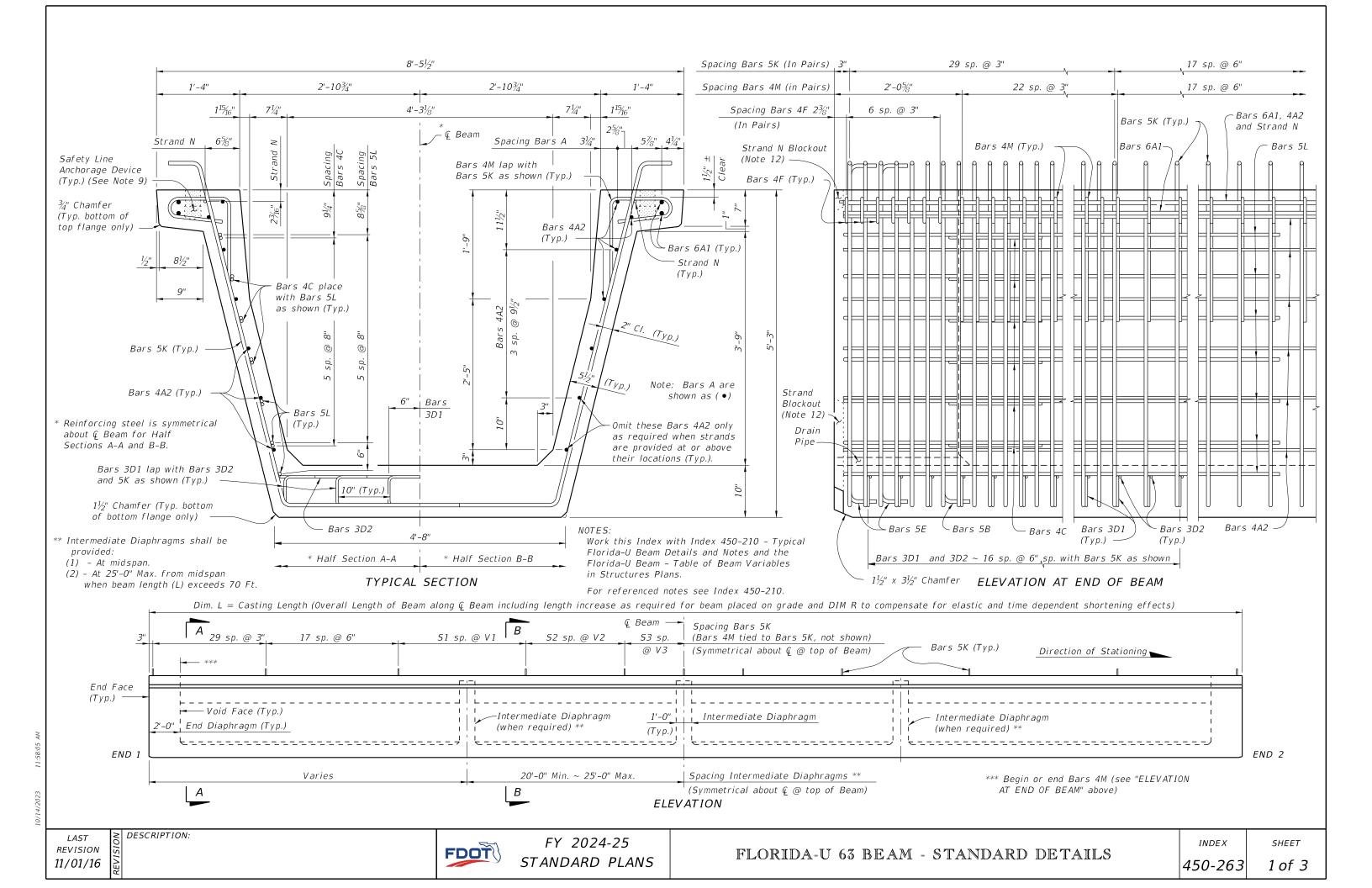
BILL OF REINFORCING STEEL FOR ONE BEAM ONLY

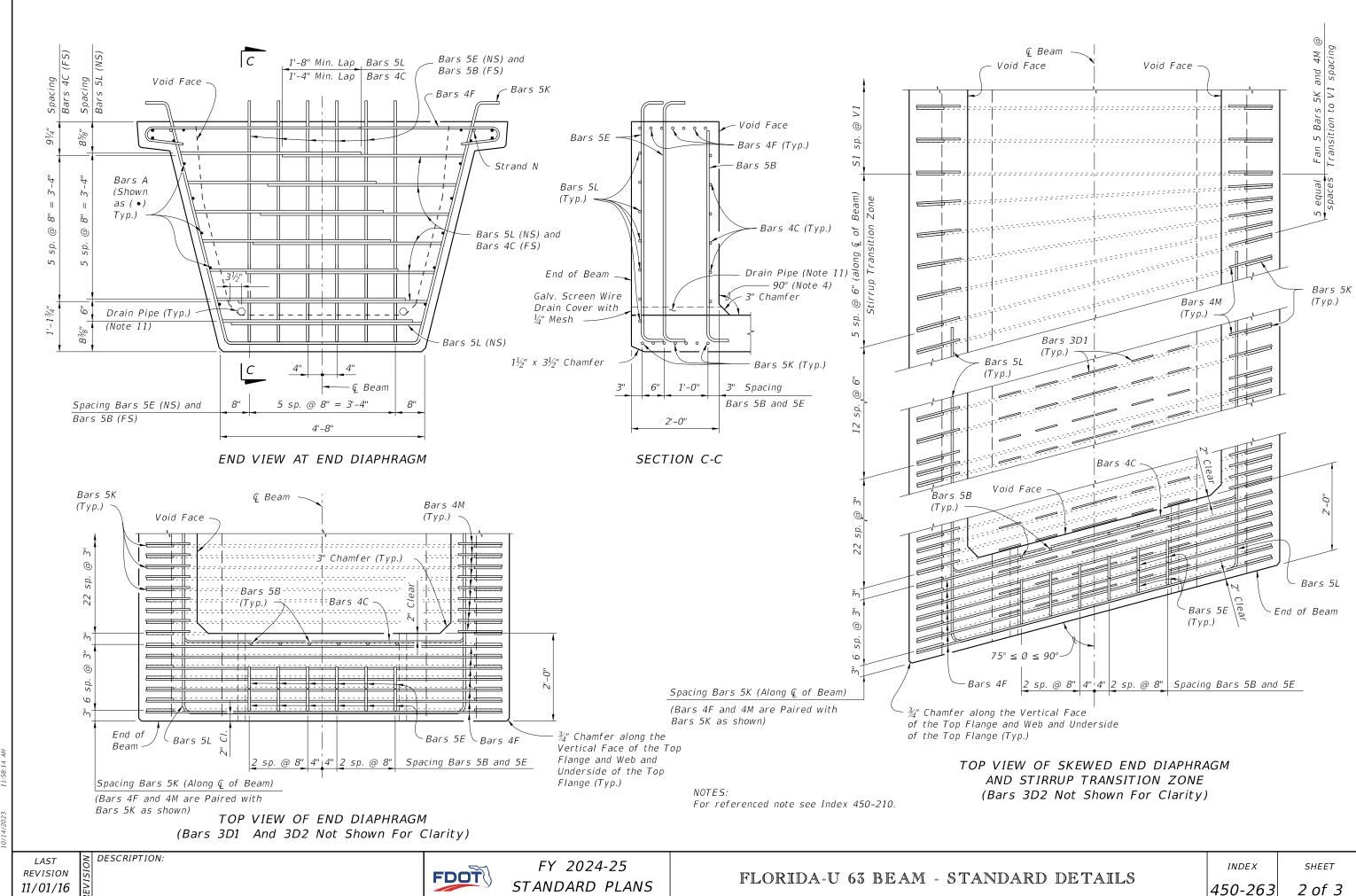
MARK	SIZE	NO. REQD.	LENGTH
A1	6	4	Dim. L - 4"
A2	4	12	Dim. L - 4"
В	5	12	4'-7"
С	4	20	5'-3"
D1	3	180	1'-6"
D2	3	30	4'-6"
Ε	5	24	5'-9"
F	4	20	6'-4"
G	4	See Table	4'-6"
Н	4	See Table	4'-9"
К	5	See Table	8'-6"
L	5	24	16'-2"
М	4	See Table	3'-11"
N	¾" Ø Strand	2	Dim. L - 3"

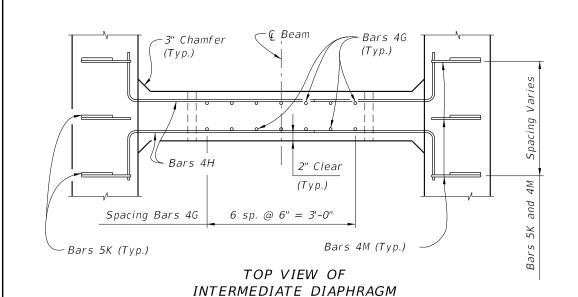


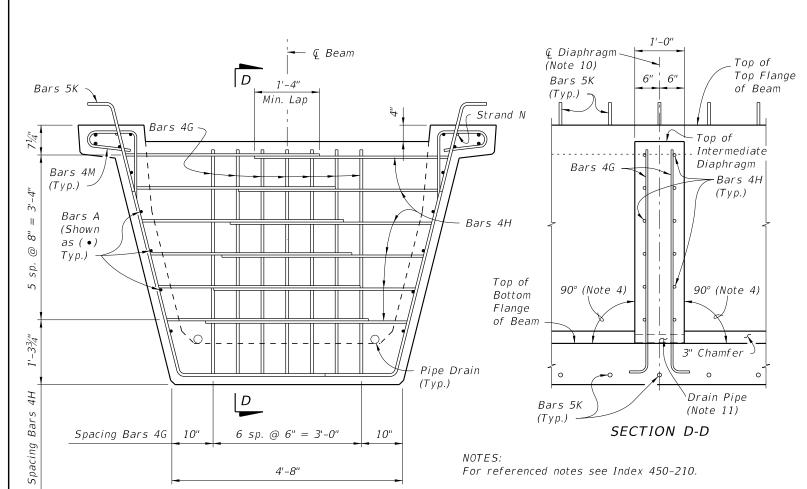


For referenced note see Index 450-210.









CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL FOR ONE BEAM ONLY MARKSIZE NO. REQD. LENGTH A1 6 4 Dim. L - 4" Dim. L - 4" A2 4 12 В 5 12 5'-4" 4 24 5'-5" D1 3 204 1'-6" Bars 3D1 D2 3 34 4'-6" Ε 5 24 6'-6" Bars 5B 4 28 F 6'-6" G 4 See Table 5'-3" L - 4" (Min. Lap Splice = 2'-0") Bars 5E 4A2 L - 4" (Min. Lap Splice = 1'-4") 4 4'-11" Н See Table 3D2 4'-6" 5 See Table 9'-21/2" 5 28 17'-8" Μ 4 See Table 3'-11" Bars 6A1, 4A2 and 3D2 Ν ¾" Ø Strand 2 Dim. L - 3" -Field Bend as Required for Skew 5'-6" 3" Ø Pin 3'-11" Bars 4C Bars 4F 3'-11" 8" Bars 4G Bars 4H 1'-0" 1'-0" 3" Ø Pin- $10\frac{1}{2}$ " -Field Bend as Required for Skew

4'-6"

Bars 5L

SECTION AT INTERMEDIATE DIAPHRAGM

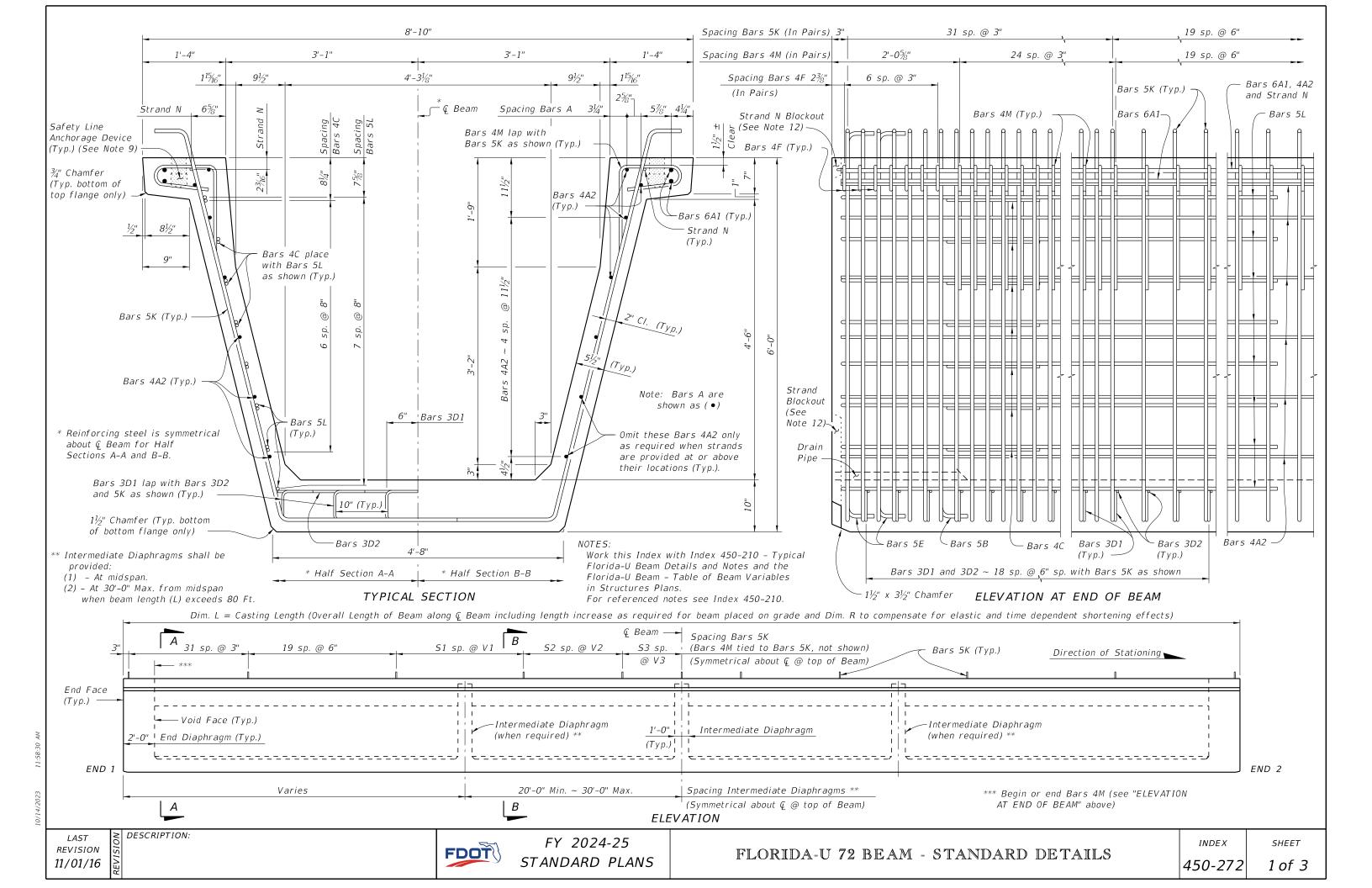
Bars 4M

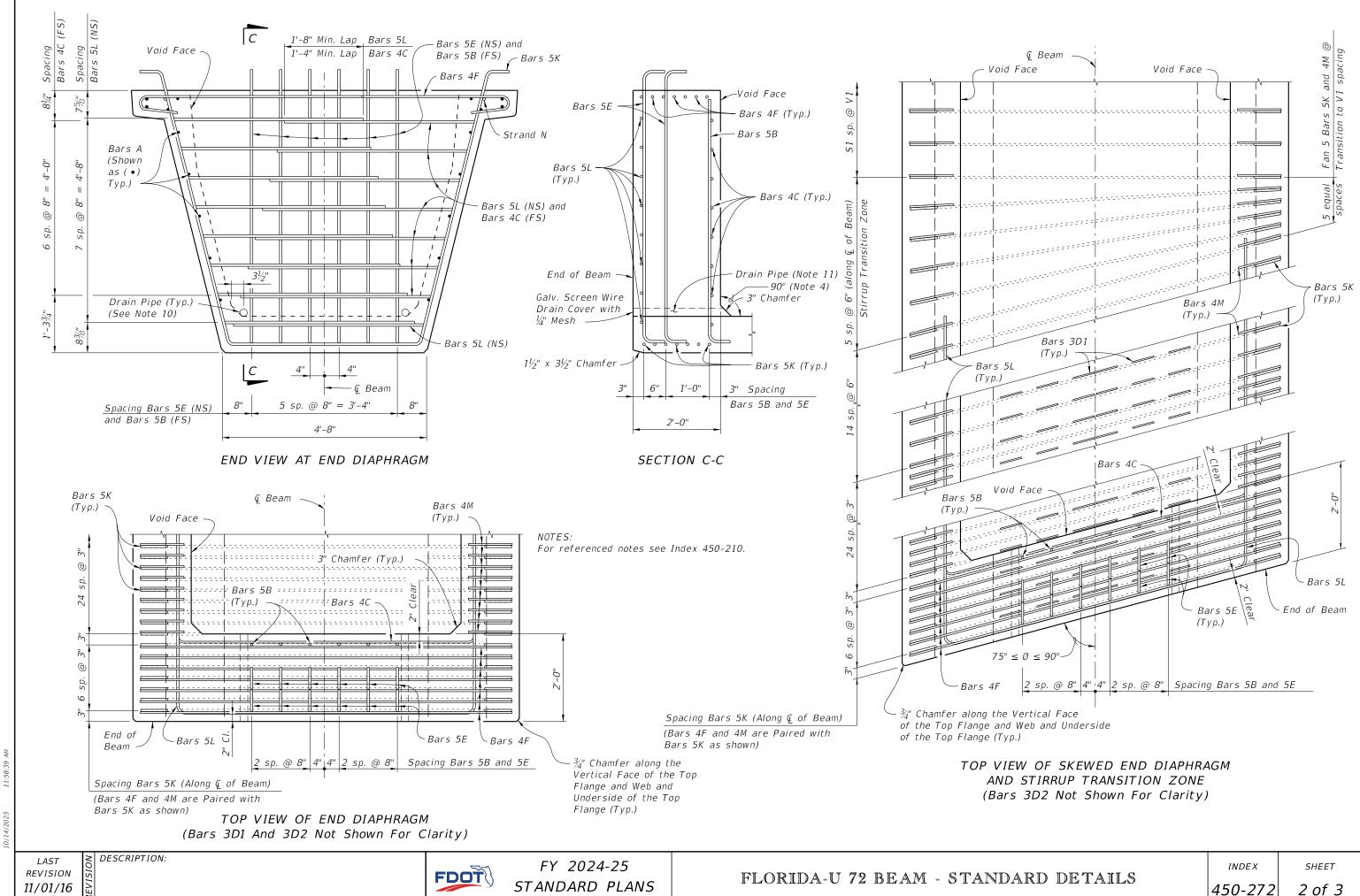
1'-4%"

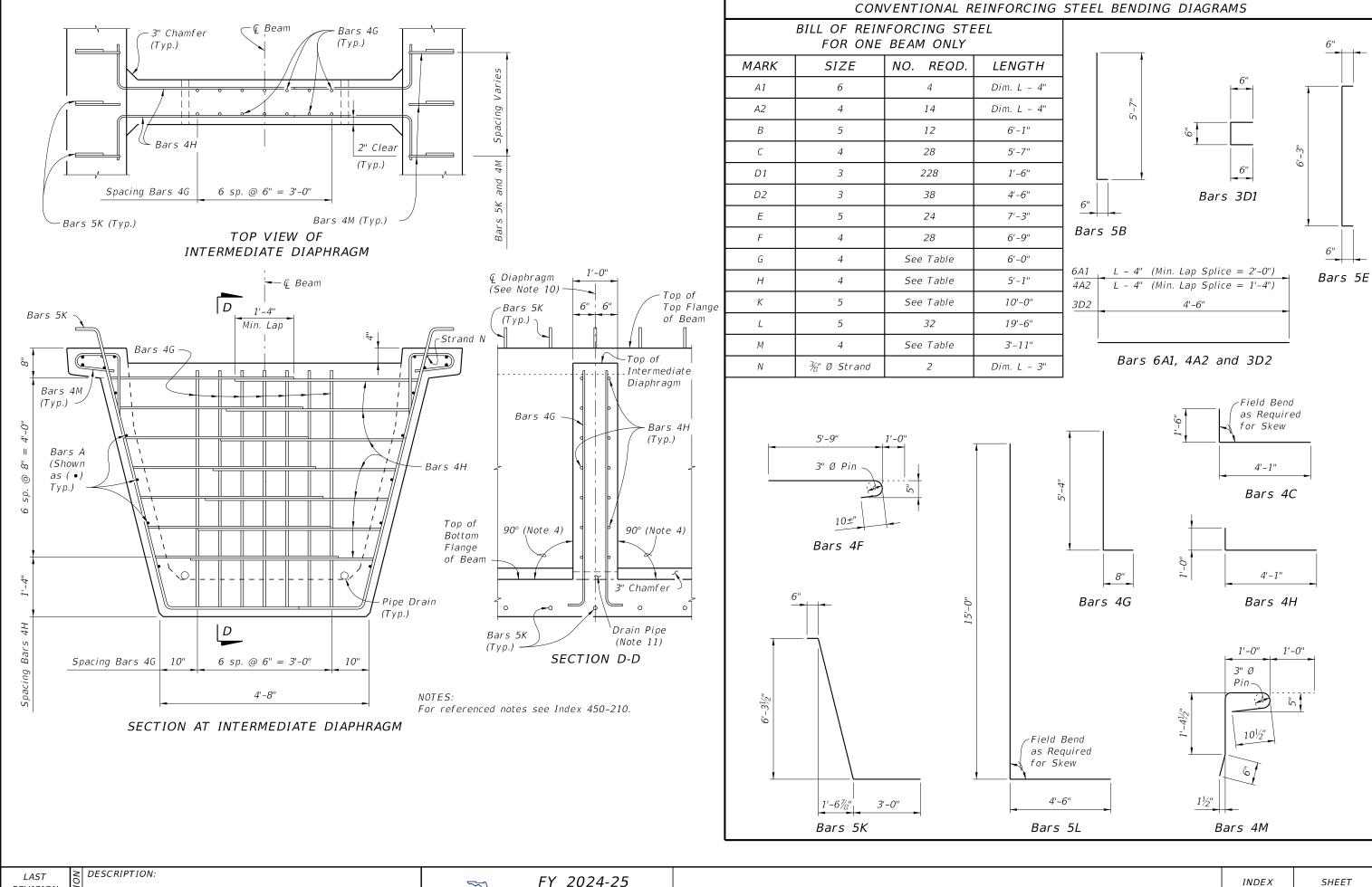
Bars 5K

3'-0"

DESCRIPTION:





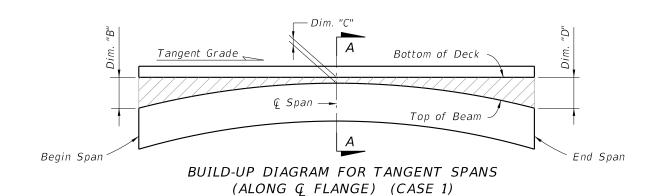


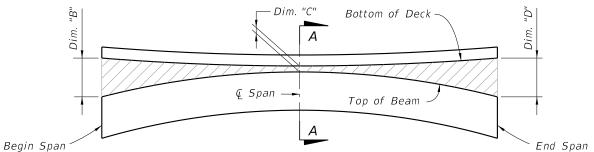
10/14/2023

REVISION

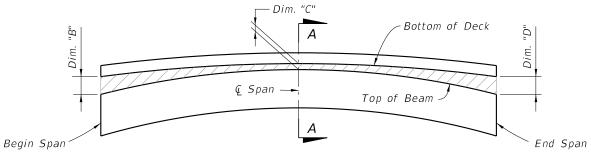
11/01/16

3 of 3

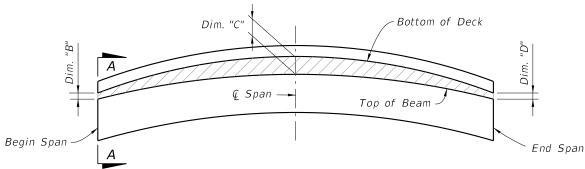




BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS
(ALONG Q FLANGE) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT Q SPAN
(ALONG Q FLANGE) (CASE 3)

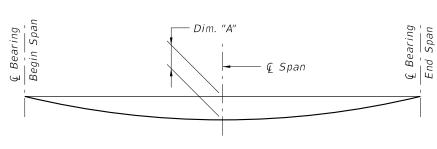


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT BEGIN OR END SPAN
(ALONG Q FLANGE) (CASE 4)

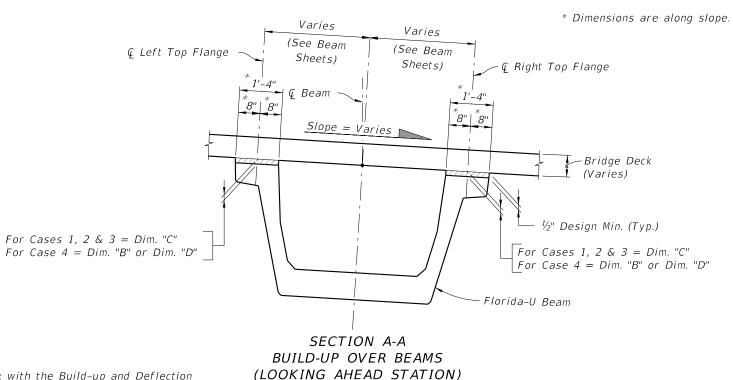
BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than $+/-\frac{1}{2}$ " from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM
(ALONG Q BEAM)



* NOTE:

Work this Index with the Build-up and Deflection
Data Table for Florida-U Beams in Structures Plans.

FLORIDA-U BEAMS
- BUILD-UP & DEFLECTION DATA

INDEX

sнеет 1 **of 1**

10/14/2023

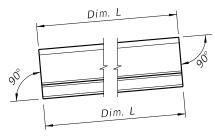
LAST REVISION 07/01/15

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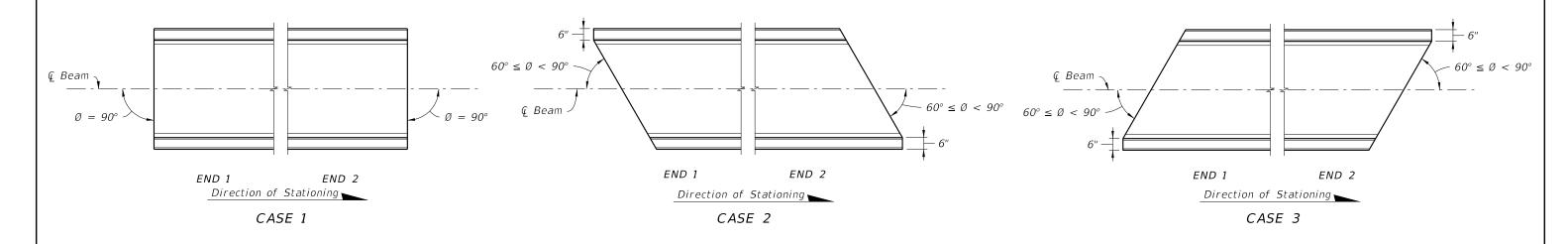
FDOT

FABRICATION NOTES

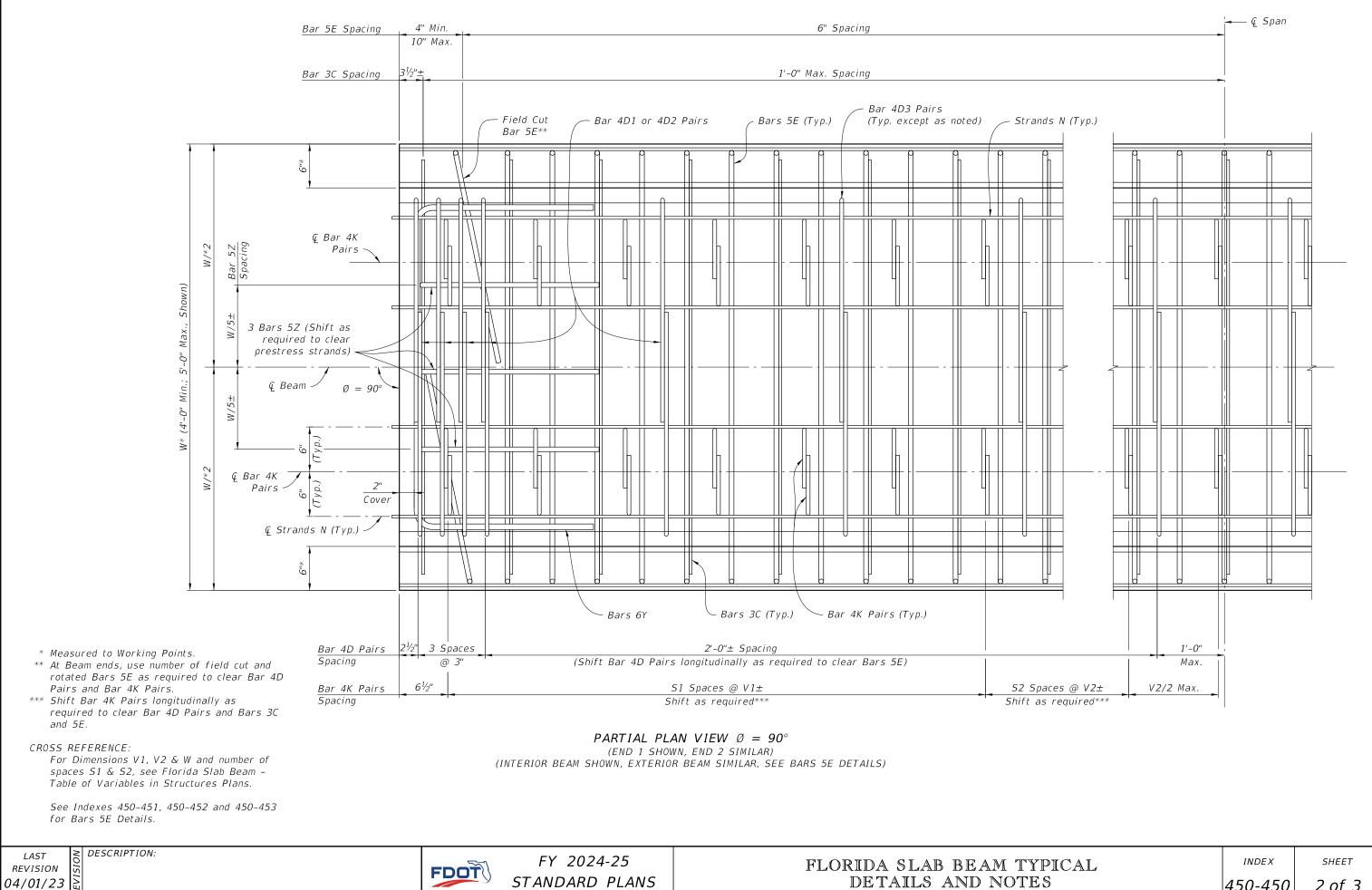
- 1. The abbreviated FSB designation for depth and width is FSB "depth" x "width", e.g. FSB 12 x 48.
- 2. All bar dimensions are out-to-out.
- 3. Strands N shall be ASTM A416, Grade 250 or 270, 3/8" Ø or larger strands, stressed to 10,000 lbs. each.
- 4. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 5. For referenced Dimensions, Angles and Case Numbers, see Florida Slab Beam - Table of Variables in Structures Plans.
- 6. Bars 4D1 & 6Y1 correspond to END 1, and 4D2 & 6Y2 correspond to END 2.
- 7. Bars 5E1 correspond to interior FSBs, and 5E2 correspond to exterior FSBs.
- 8. Rake the top surface of the Slab Beams transversely to provide a roughened surface with $\frac{1}{4}$ " amplitude.
- 9. Embedment of Safety Line Anchorage Devices are permitted to accommodate full protection systems. See shop drawings for details and spacings.



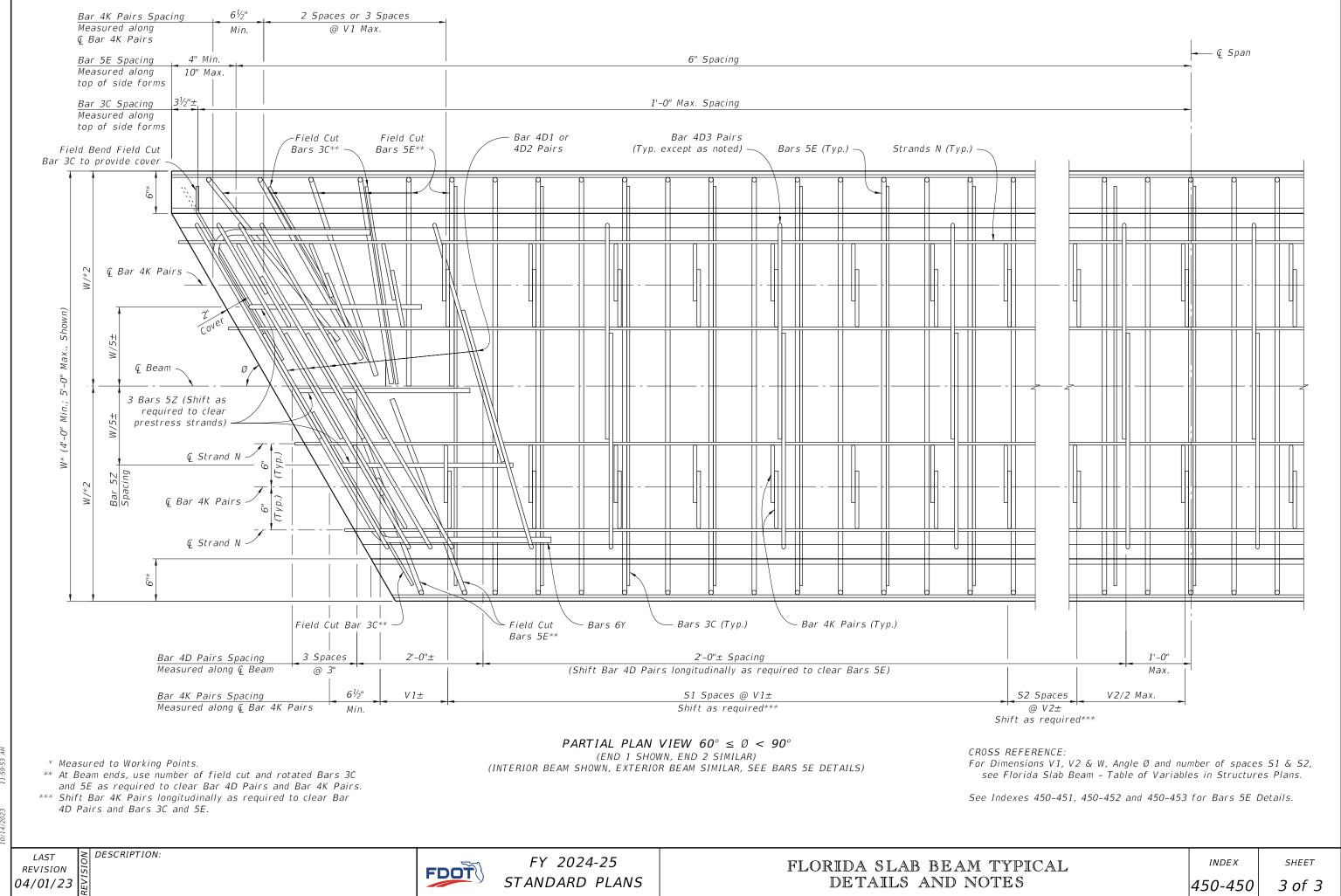
SCHEMATIC SIDE ELEVATION OF BEAM (Beam on a Positive Grade shown; Beam on a Negative Grade or Horizontal Grade similar.)

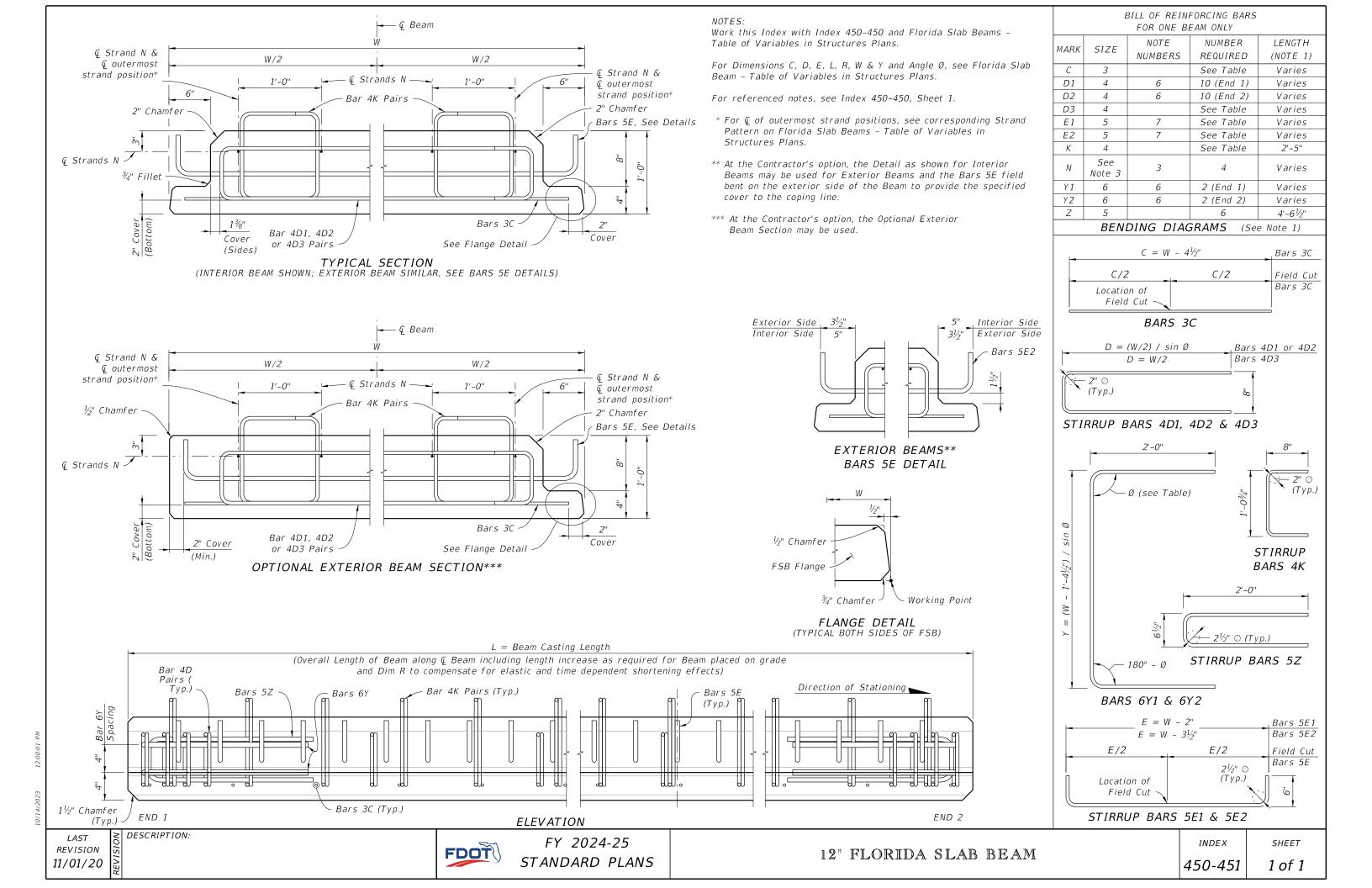


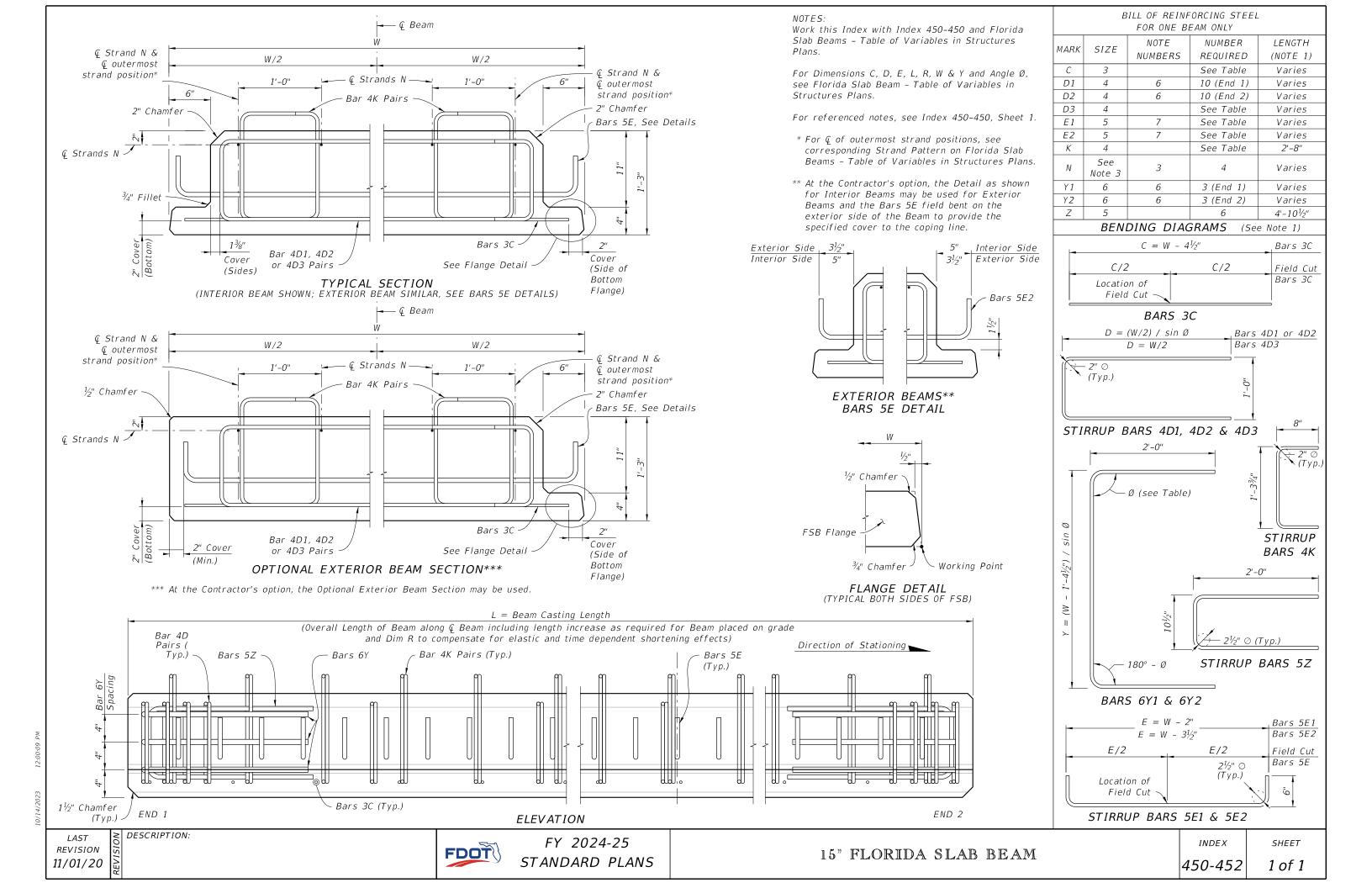
SCHEMATIC PLAN VIEWS AT BEAM ENDS

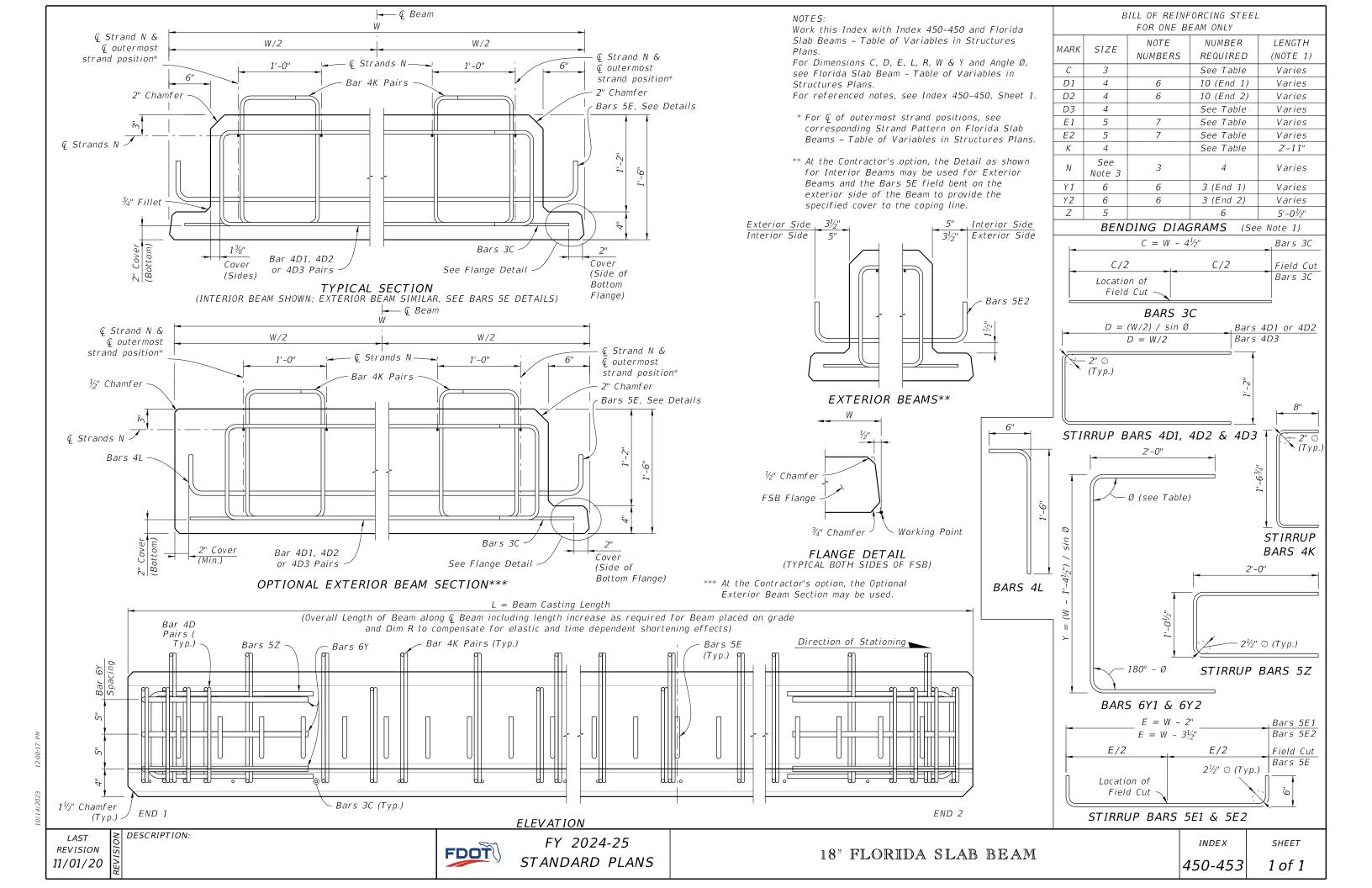


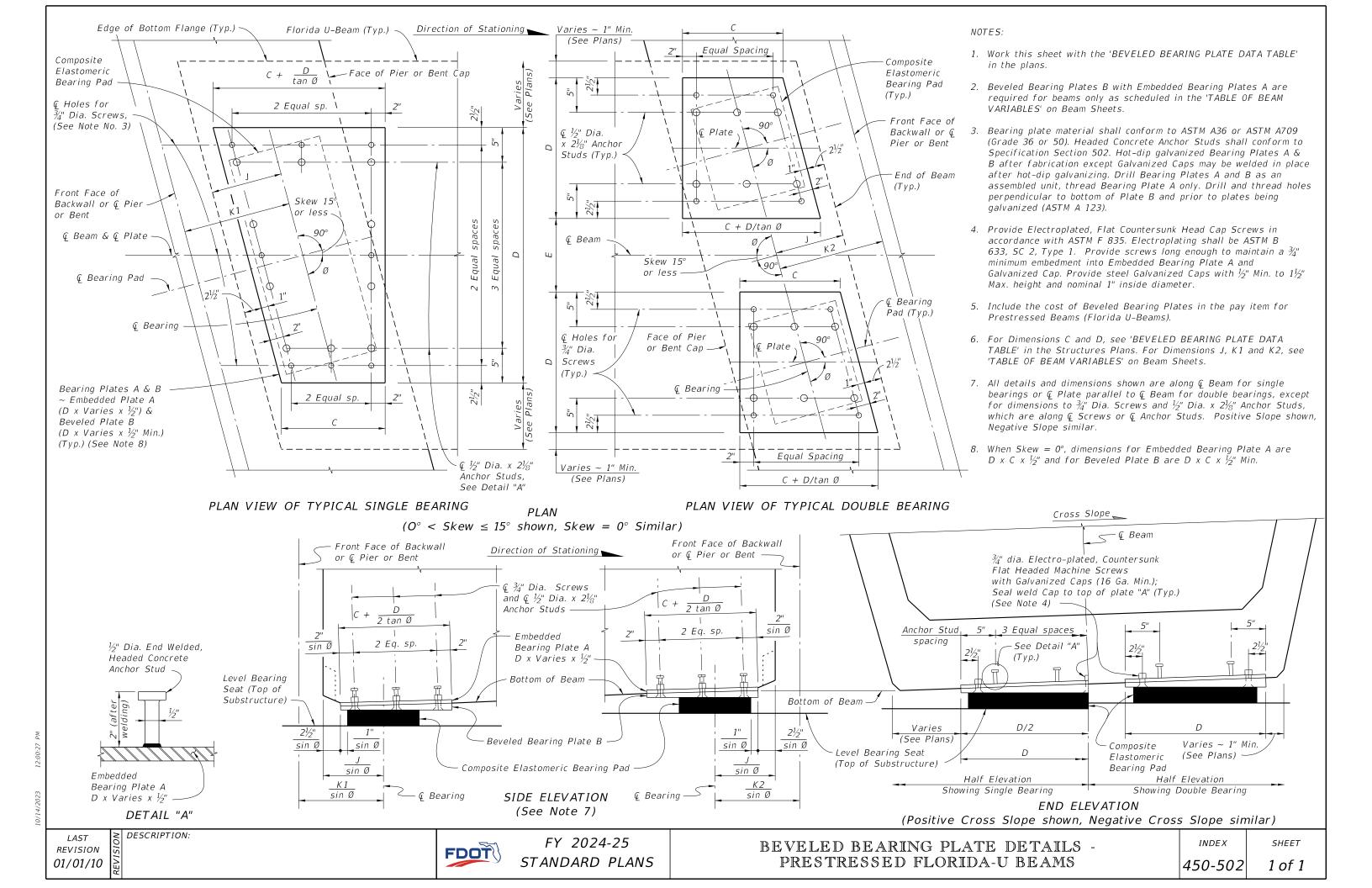
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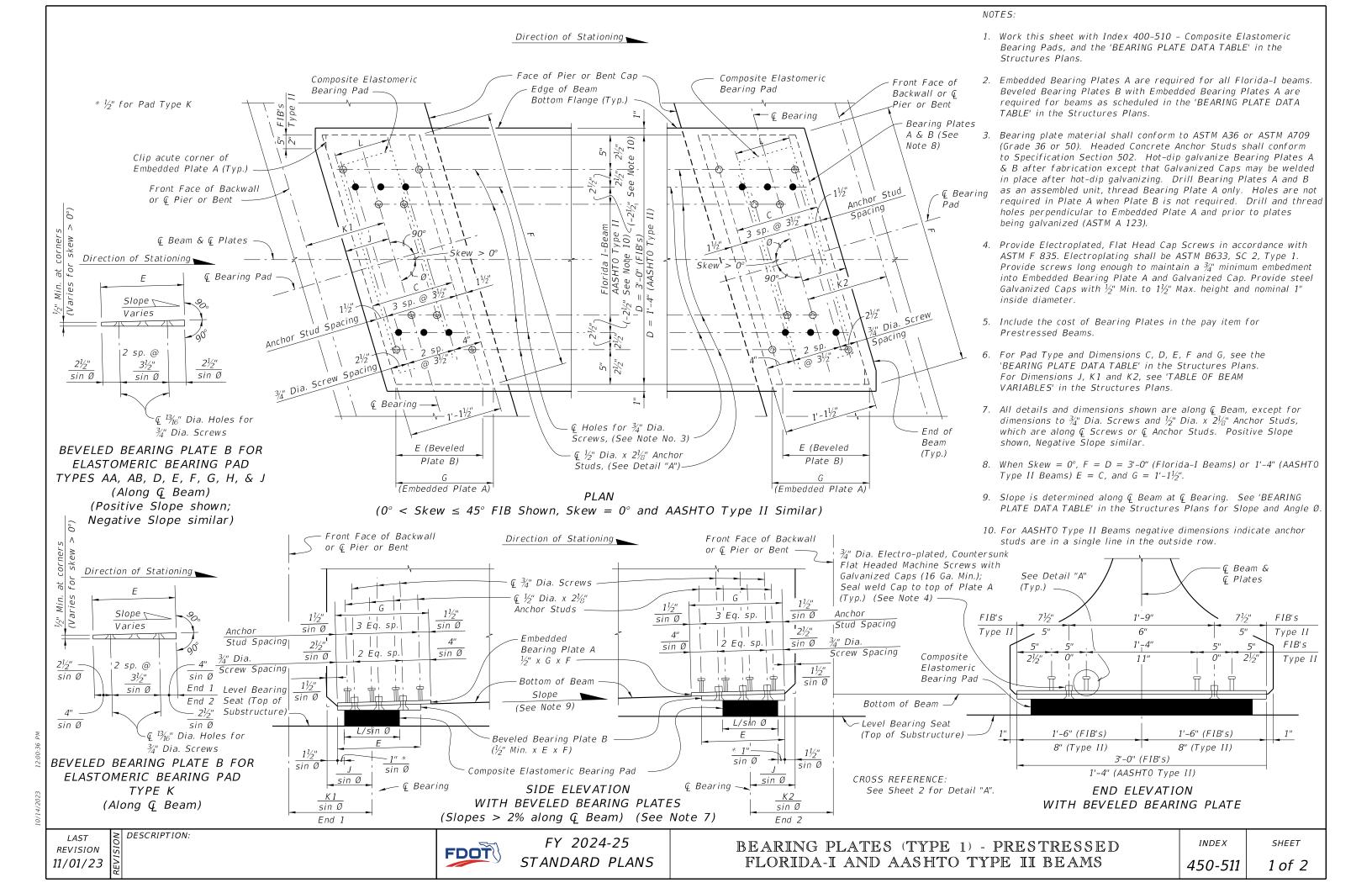


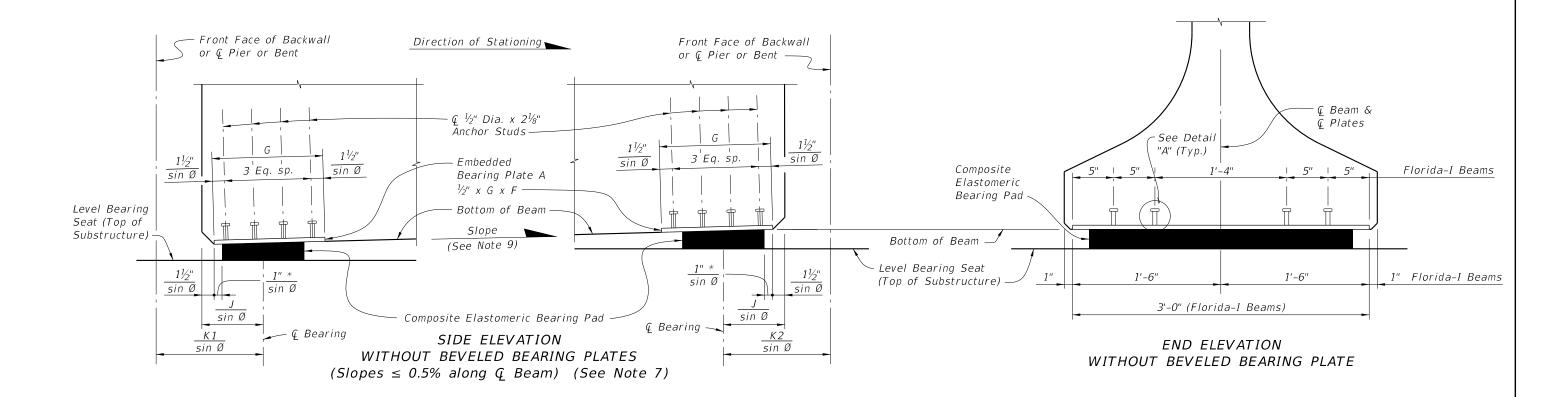




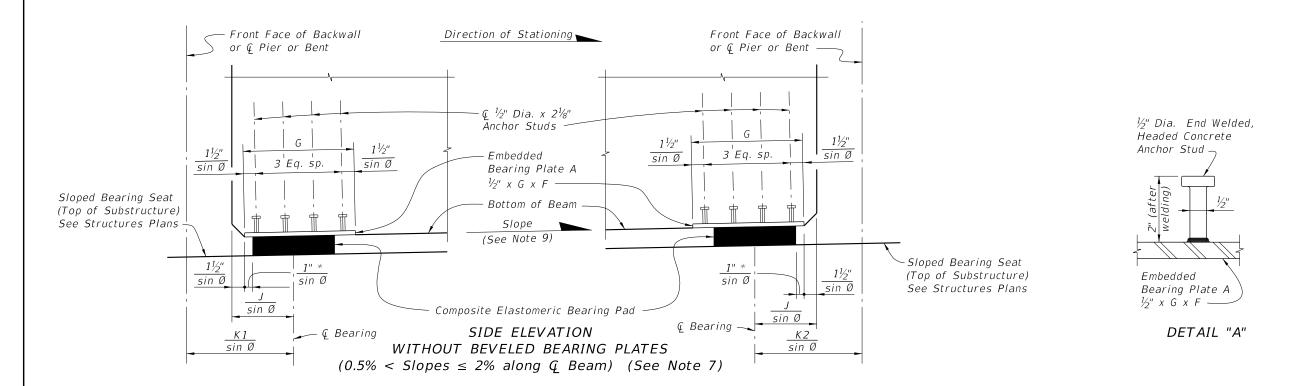








* 1/3" Pad Type K



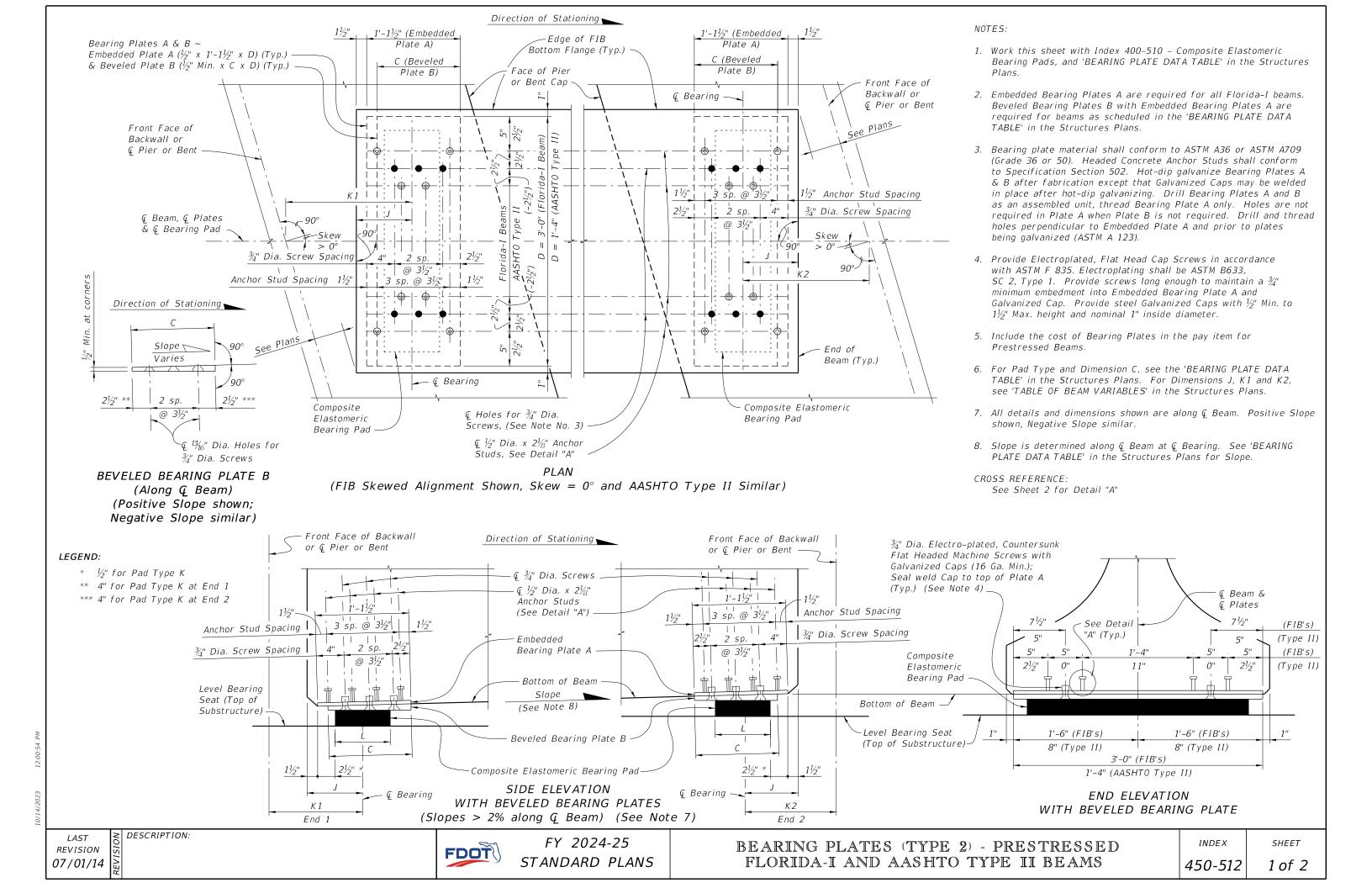
CROSS REFERENCE: See Sheet 1 for Notes.

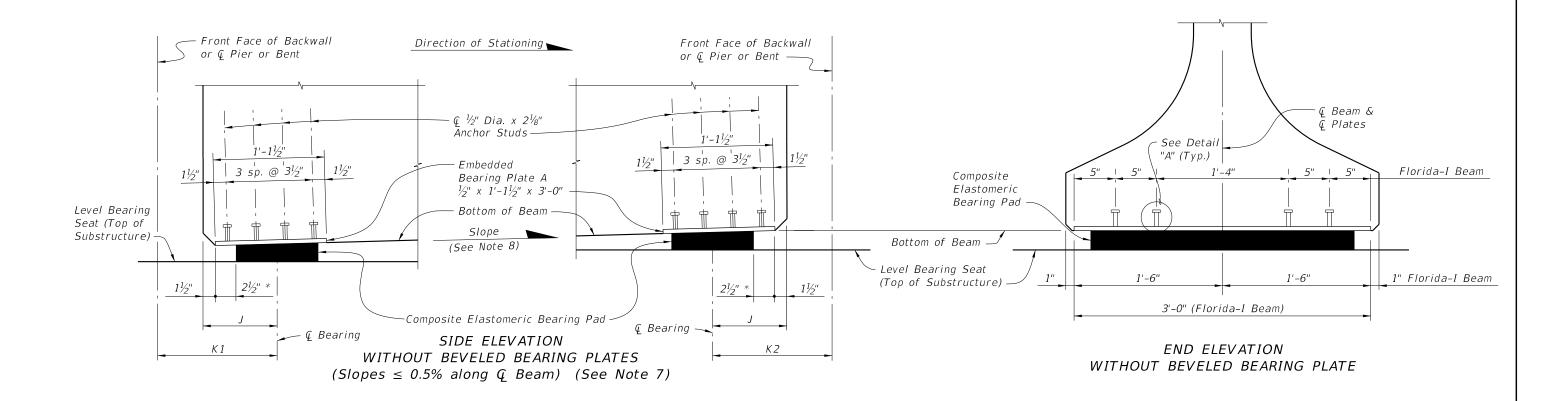
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REVISION 11/01/20

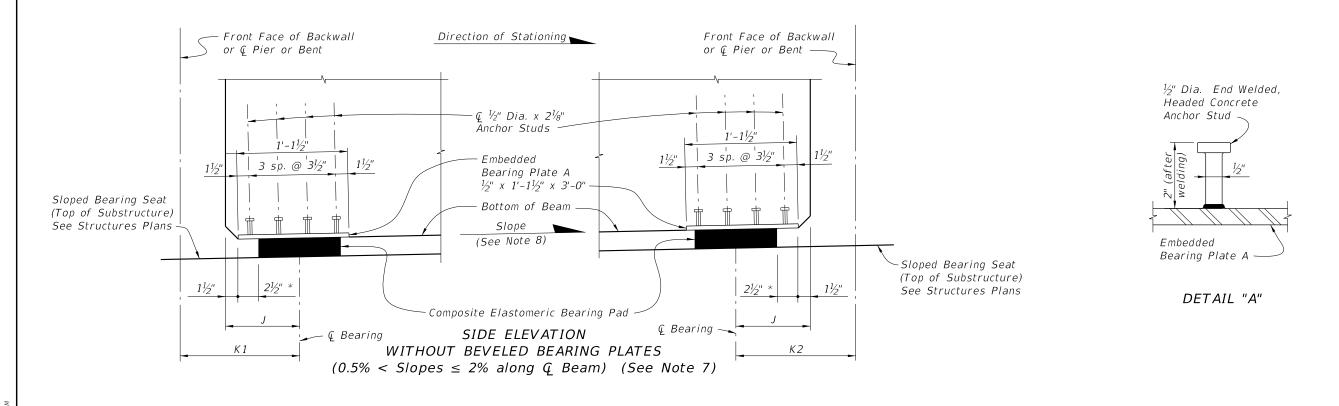
FDOT

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* ½" for Pad Type K



CROSS REFERENCE: See Sheet 1 for Notes.

LAST REVISION 07/01/14

DESCRIPTION:

FDOT

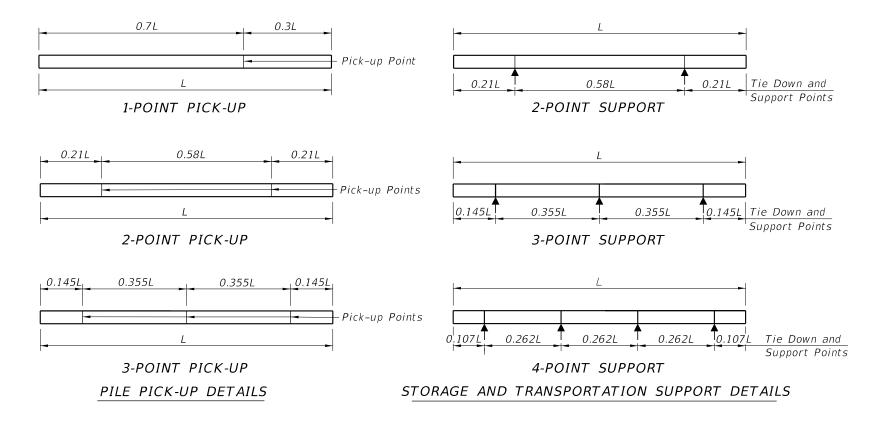
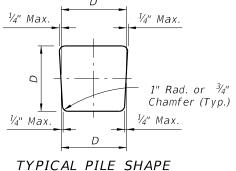
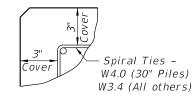


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = 0	Square	e Pile	Size	(inches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point				
	69	75	85	98	124	2, 3, or 4 point	2 Point				
	99	107	121	140	178	3 or 4 point	3 Point				





TYPICAL PILE SHAPE DETAIL SHOWING
FOR MOLD FORMS TYPICAL COVER

PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V, except use Class VI for High Moment Capacity Pile (Index 455-031).
 - B. High Capacity Splice Collar: Class V.
 - C. See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
 - B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
 - A. Bars: Meet the requirements of Specification Section 415.
 - B. Prestressing Strands: Meet the requirements of Specification Section 933.
 - C. Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
 - B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

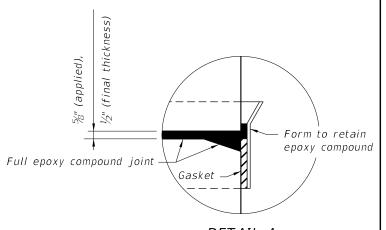
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455-001

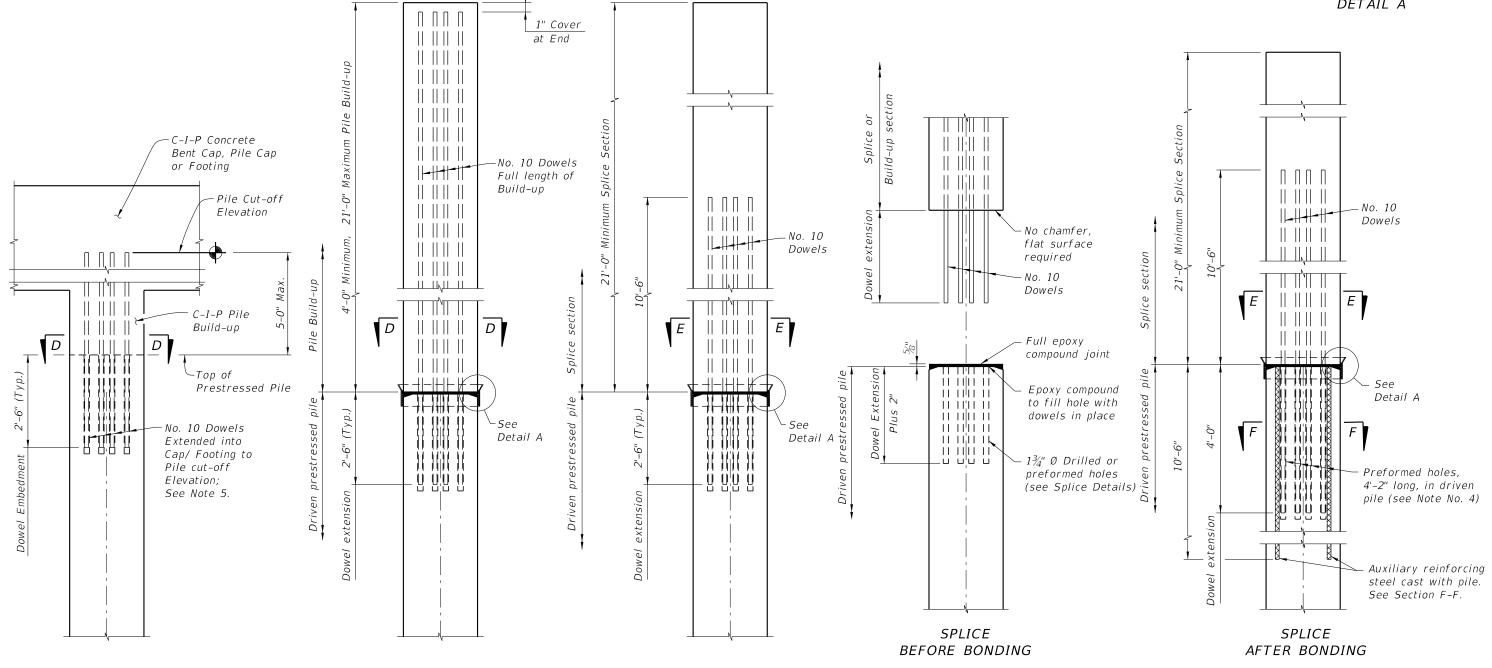
01 1 of 1

NOTES:

- 1. For Sections D-D, E-E, & F-F see Index 455-012 thru 455-030 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- 3. When pile splices are necessary due to shipping and handling limitations, use the "Drivable Planned Prestressed Precast Splice Detail" or Mechanical Pile Splices on the Approved Products List (APL).
- 4. When preformed dowel holes are used, continue the 1" spiral tie pitch to 4'-0" below the head of the pile, See Index 455-018, 455-020 & 455-024. For preformed holes; use either removable preforming material or stay-in-place corrugated galvanized steel ducts meeting ASTM Specification A653, Coating Designation G90, 26 gauge. Use 2" diameter ducts with a minimum corrugation (rib) height of 0.12 in. fabricated with either welded or interlocked seams. Galvanizing of welded seams is not required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 10 Dowels into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.



DETAIL A



DRIVABLE UNPLANNED

PRESTRESSED PRECAST

PILE SPLICE DETAIL

LAST

11/01/22

UNPLANNED

REINFORCED C-I-P

PILE BUILD-UP DETAIL

NON-DRIVABLE UNPLANNED

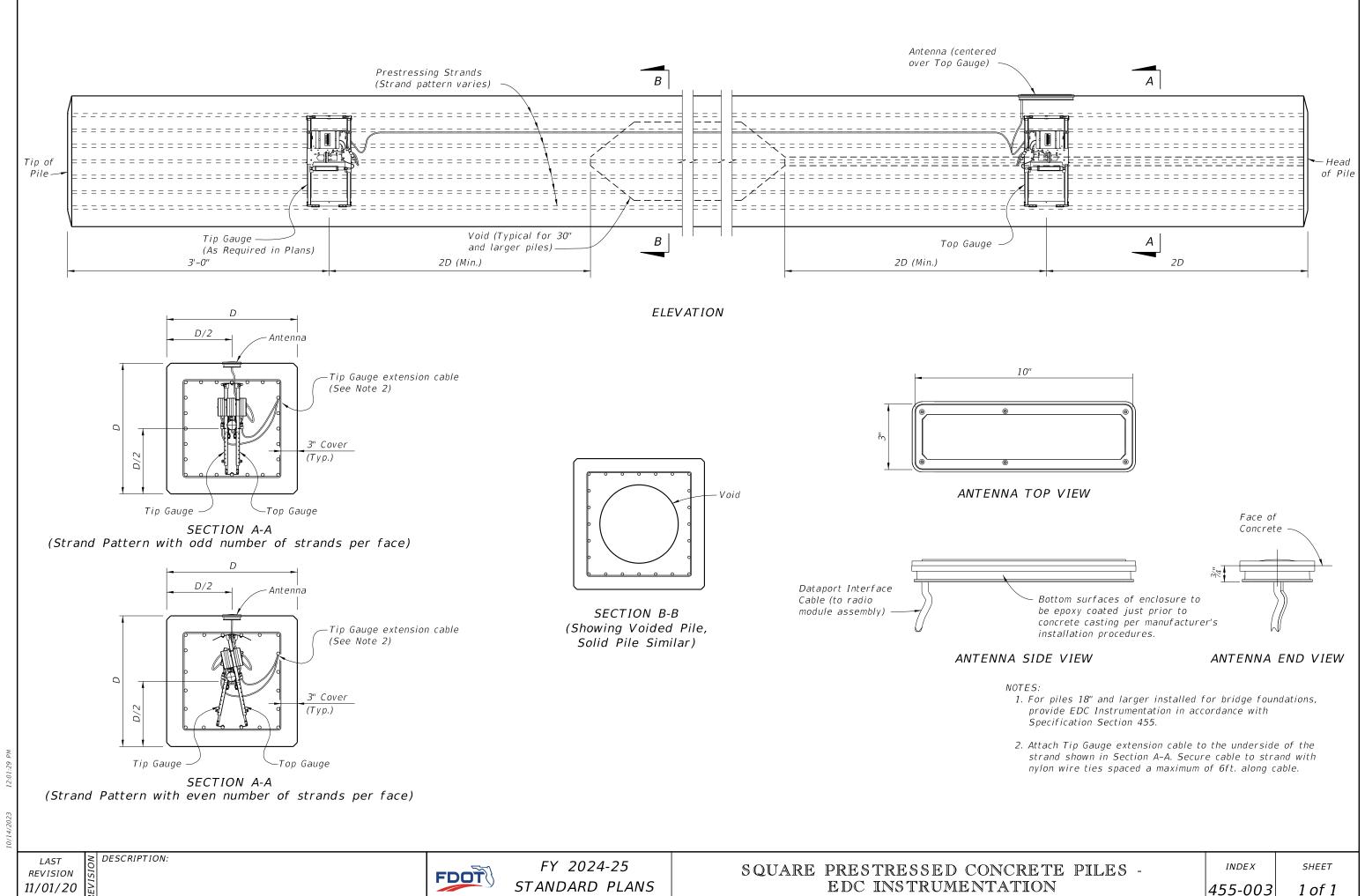
REINFORCED PRECAST

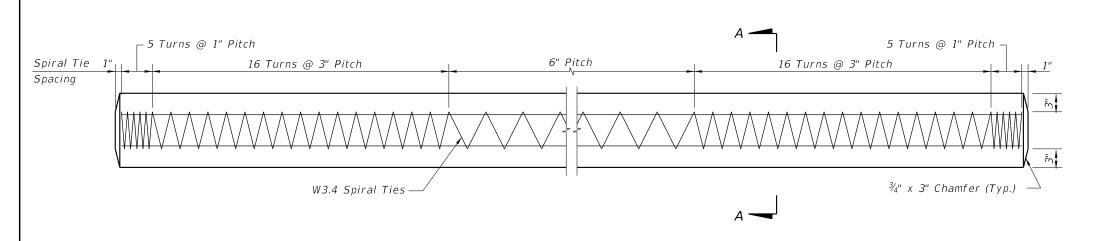
PILE BUILD-UP DETAIL

DRIVABLE PLANNED

PRESTRESSED PRECAST

PILE SPLICE DETAIL





ALTERNATE STRAND PATTERNS

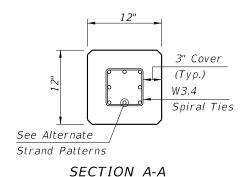
4 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

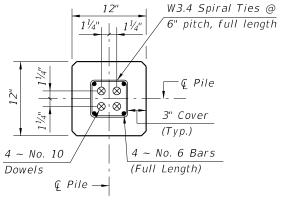
 $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 25 kips

 $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 24 kips

 $8 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 23 kips

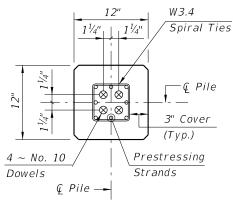
 $12 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 16 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

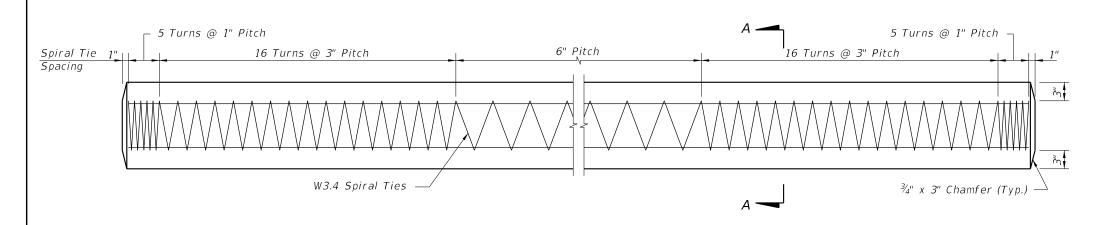
(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

NOTES:

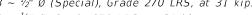
- 1. Work this Index with Index 450-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 -Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



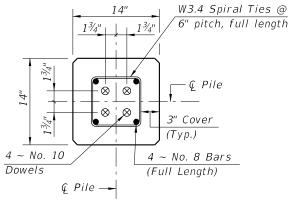
ALTERNATE STRAND PATTERNS

- 8 ~ 0.6" Ø, Grade 270 LRS, at 33 kips
- $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 31 kips
- $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips



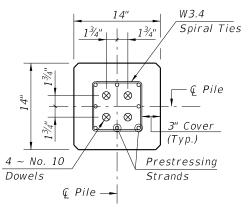
 $12 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips

 $16 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 16 kips



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



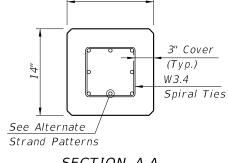
SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

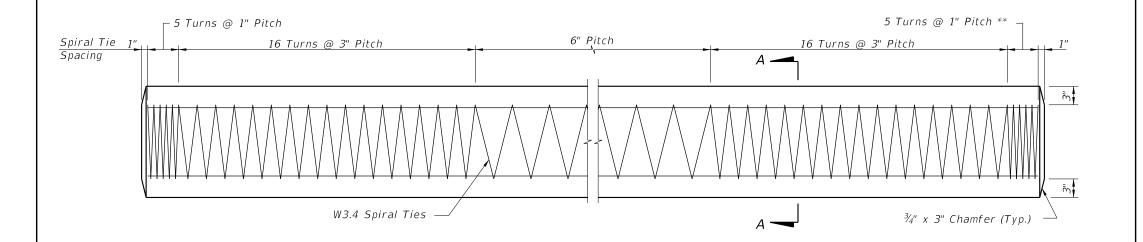
PILE SPLICE REINFORCEMENT DETAILS

- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

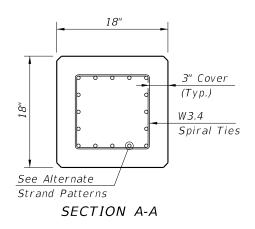
Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



SECTION A-A



ELEVATION



ALTERNATE STRAND PATTERNS

 $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

 $20 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips

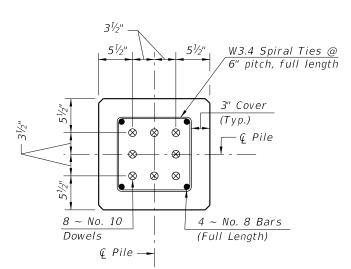
 $24 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 17 kips

12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips

$16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips

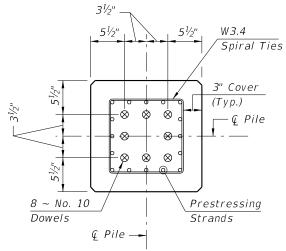
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



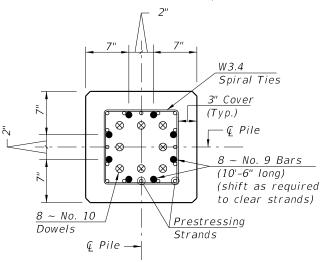
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

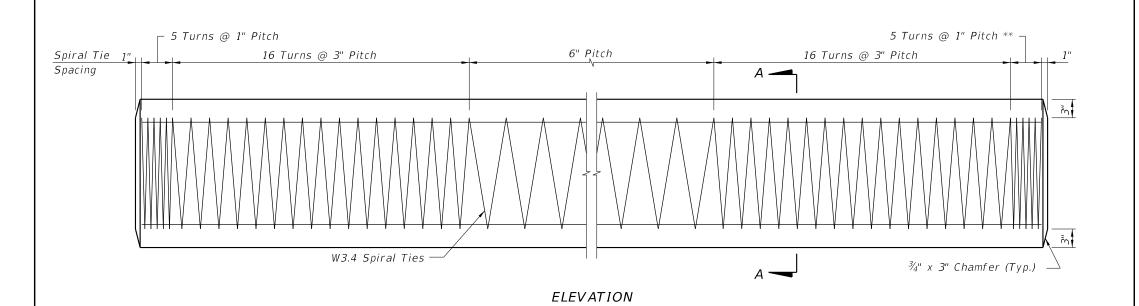
(See Drivable Prestressed Precast Splice Detail)

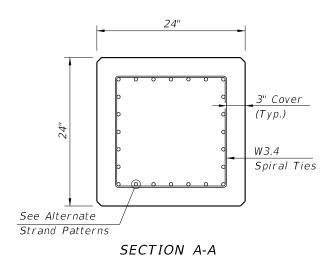


SECTION F-F

(See Drivable Preplanned Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS





ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

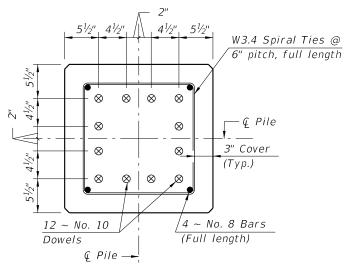
 $20 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

 $24 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips

NOTES:

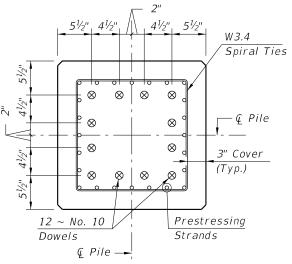
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



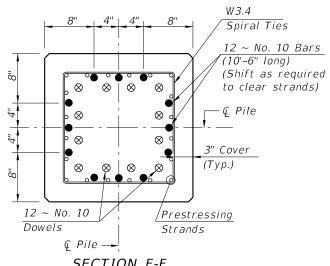
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

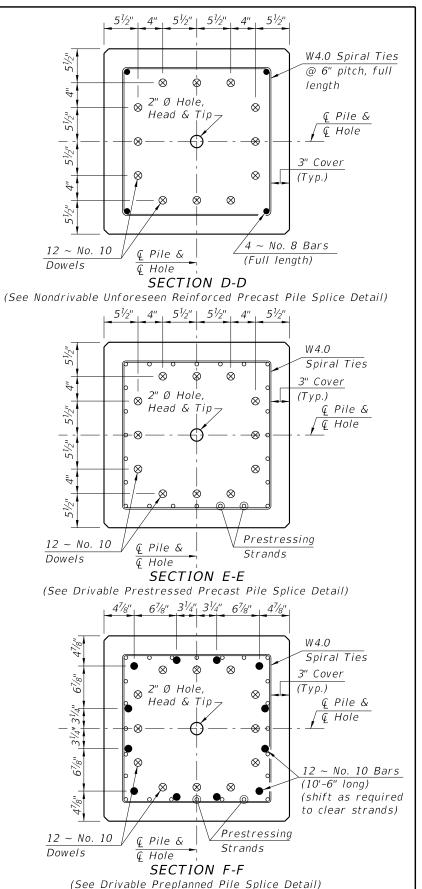
(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

- by the Engineer.
- 4. Work this Index with Index 455-001 Notes and Details for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.

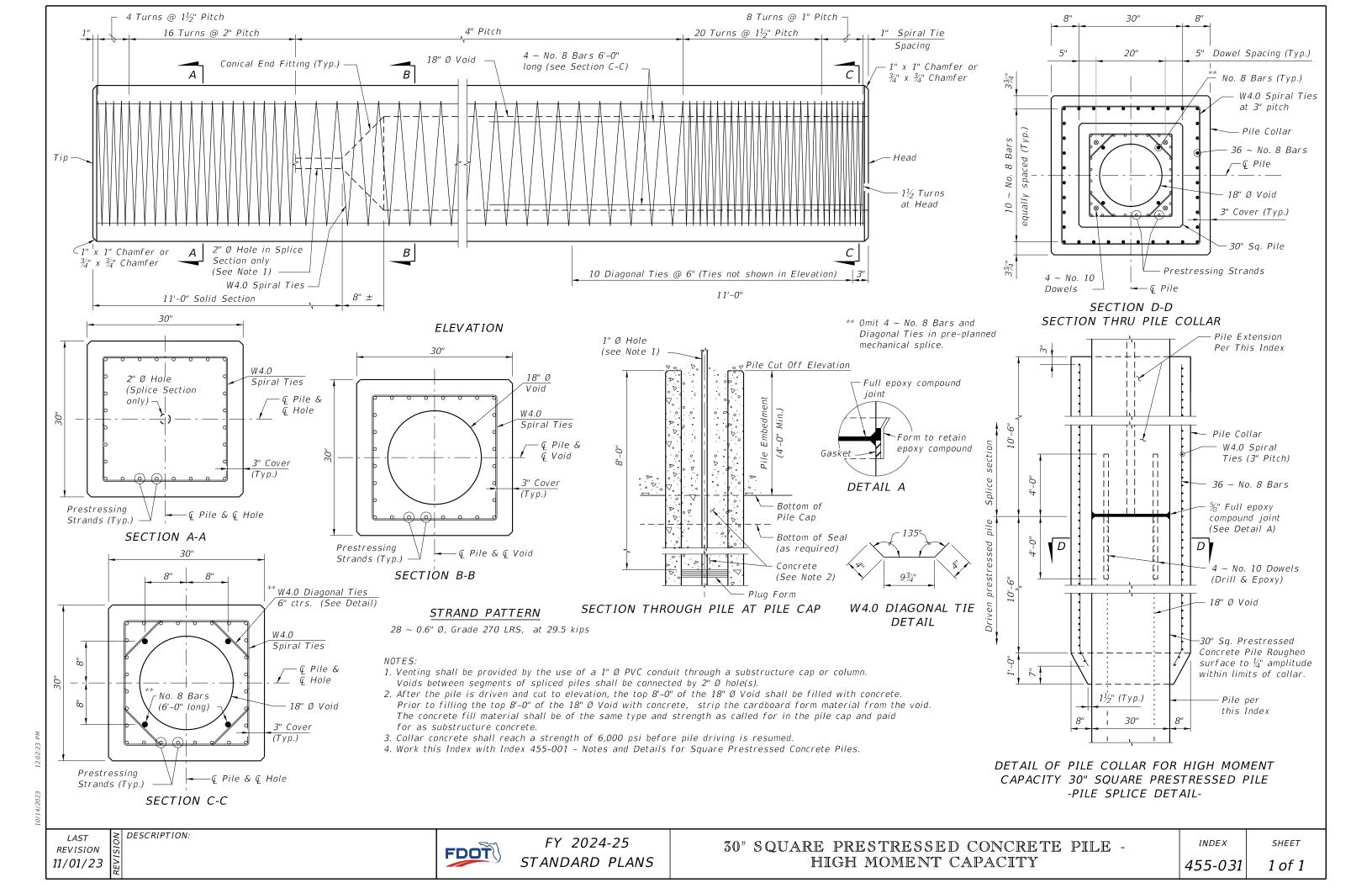


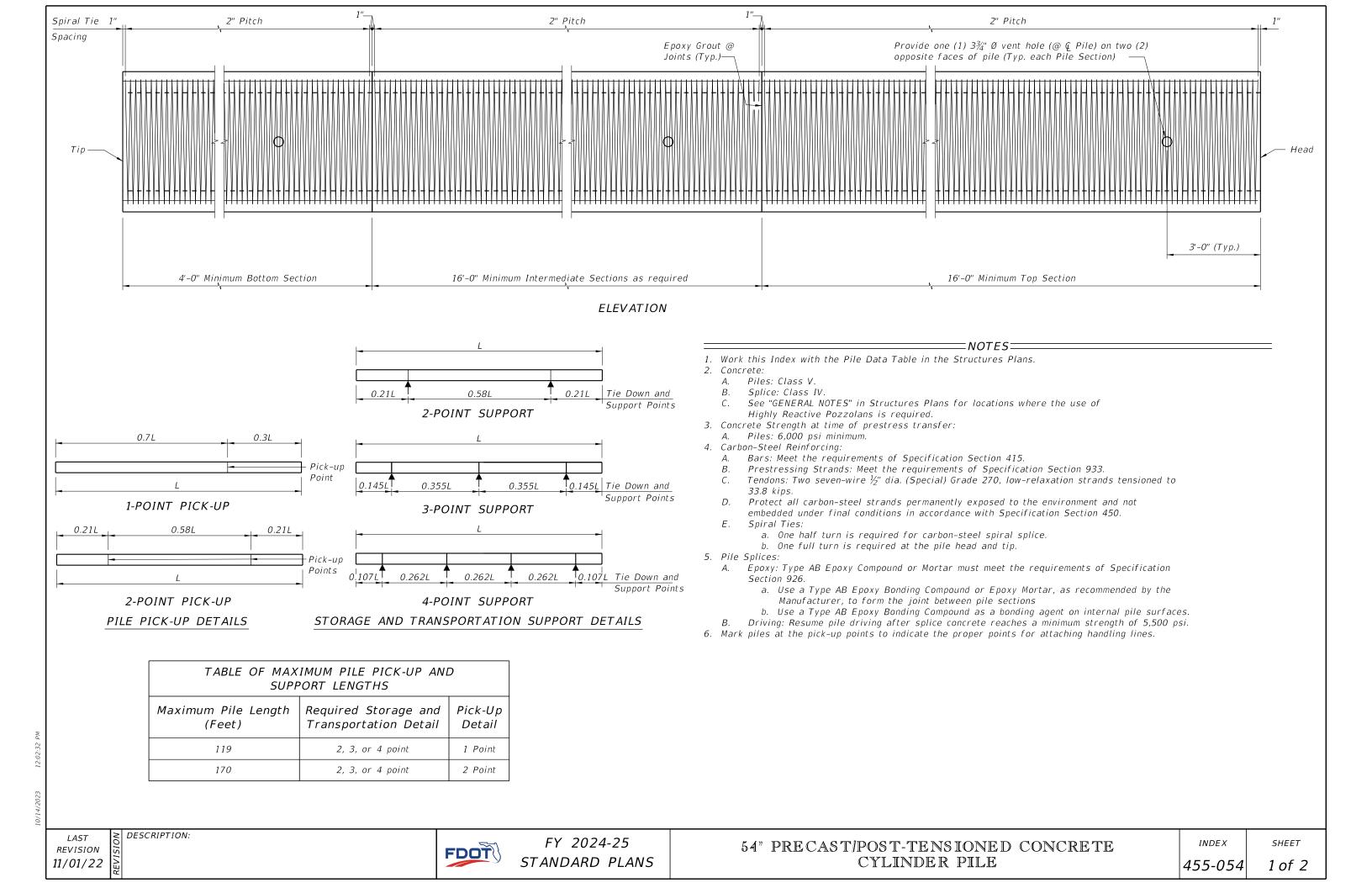
PILE SPLICE DETAILS

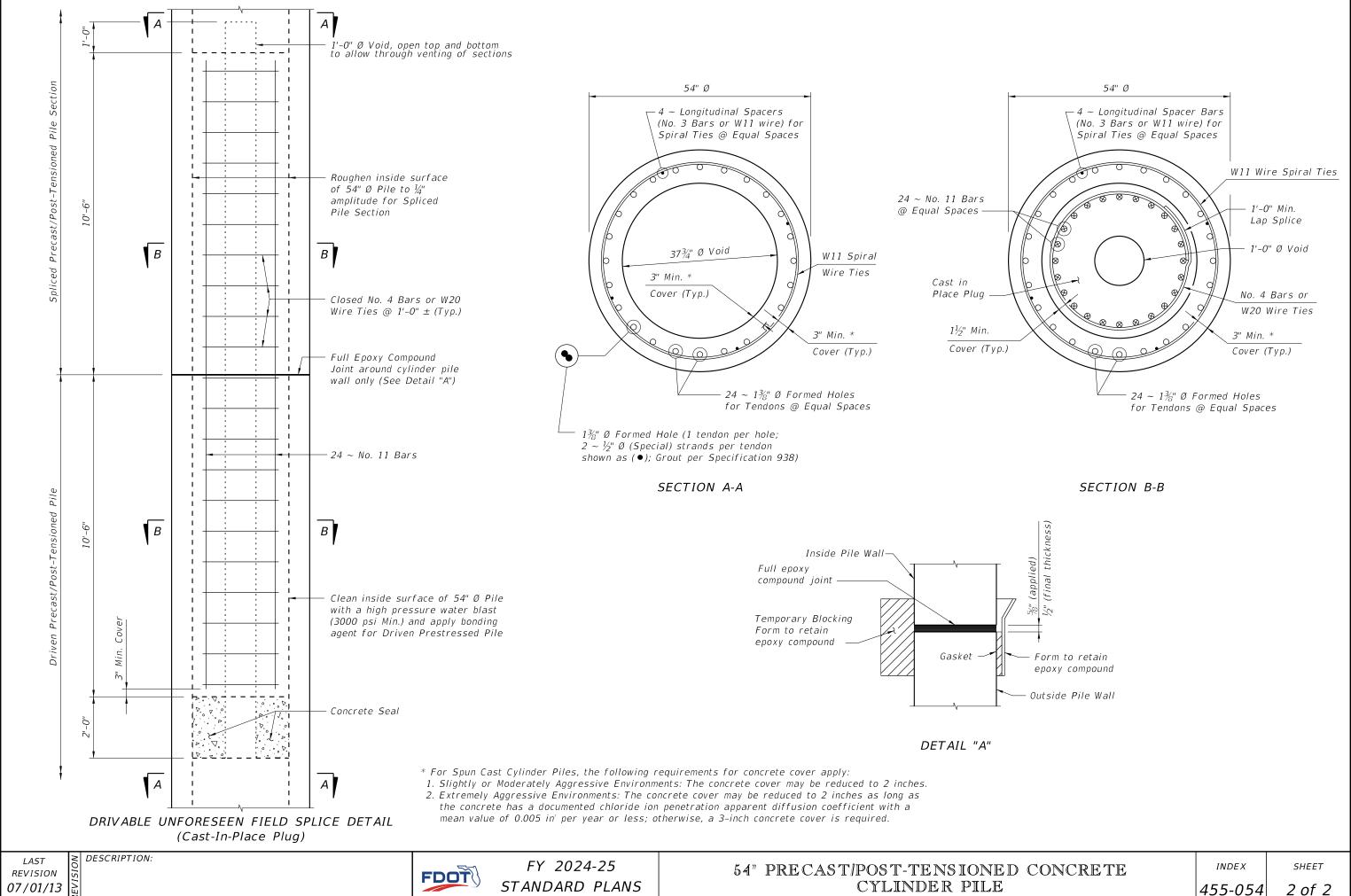
REVISION 11/01/22

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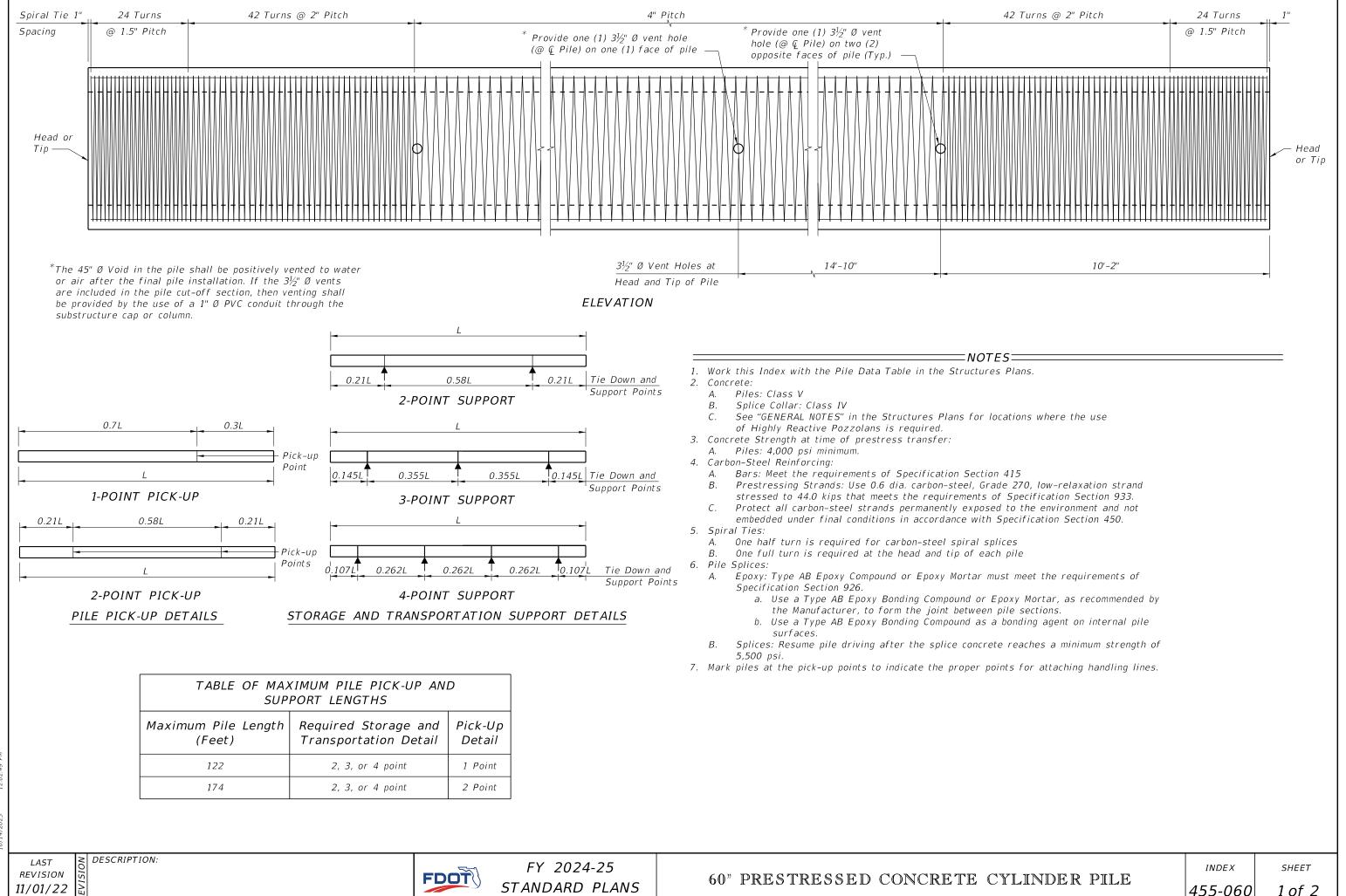
FDOT



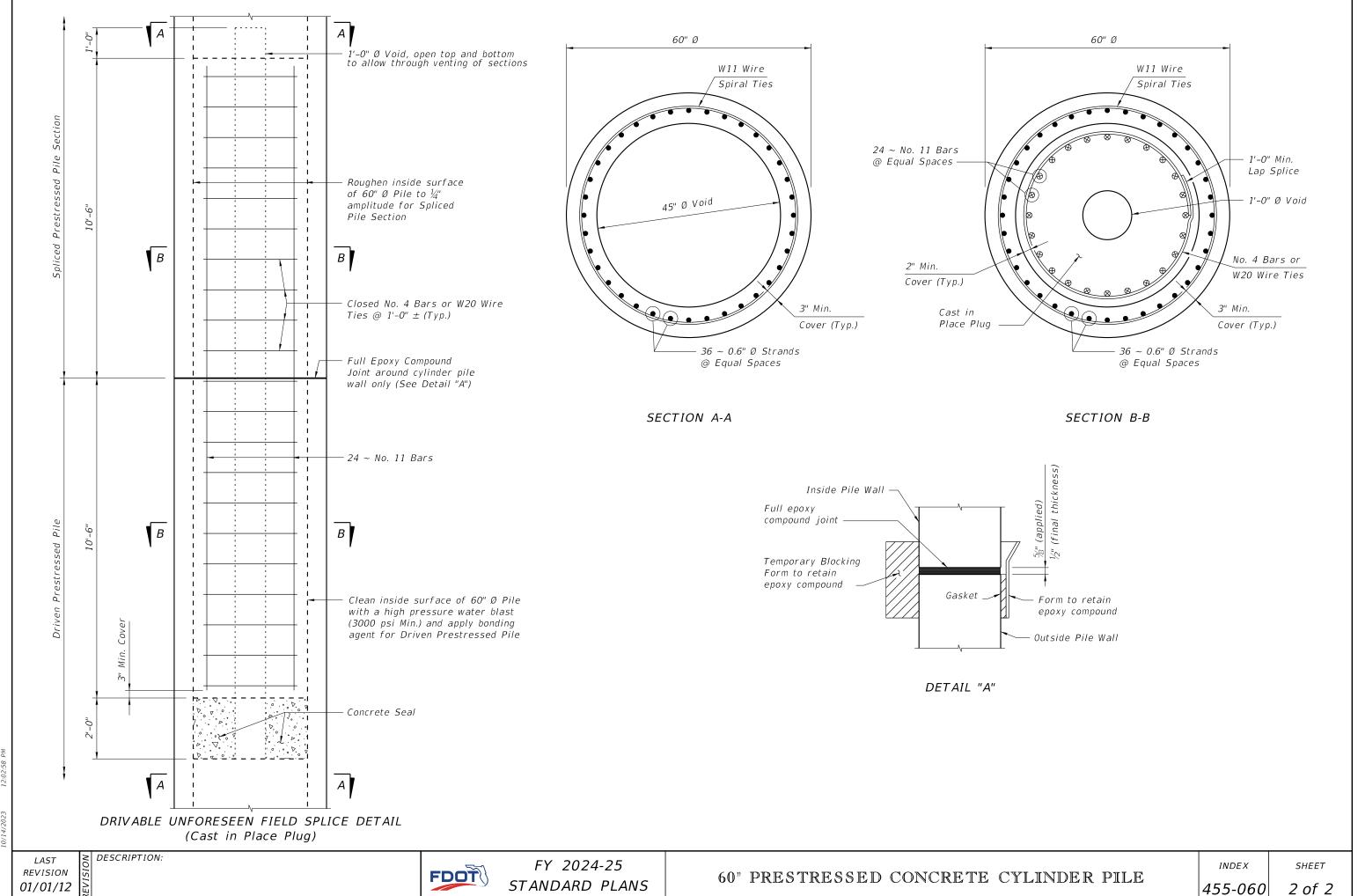




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11/01/22



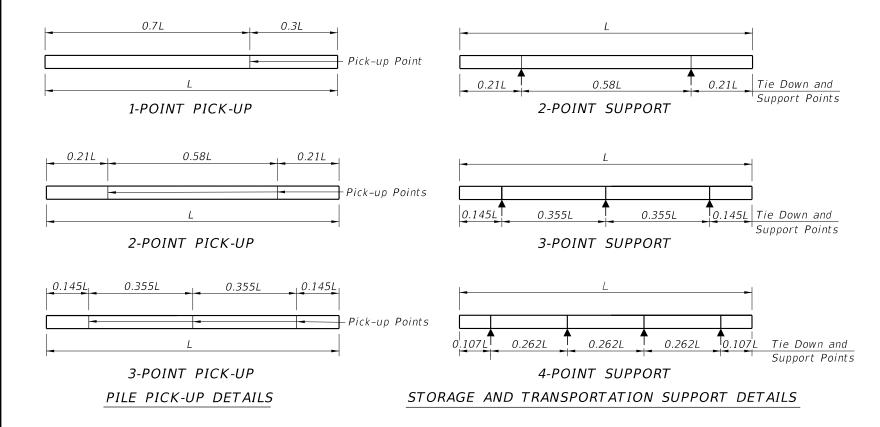
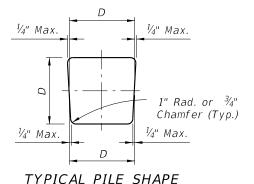
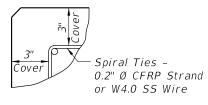


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = S	Square	Pile S	ize (in	nches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum	48	52	59	68	87	2, 3, or 4 point	1 Point				
Pile Length	69	75	85	98	124	2, 3, or 4 point	2 Point				
(Feet)	99	107	121	140	178	3 or 4 point	3 Point				



FOR MOLD FORMS



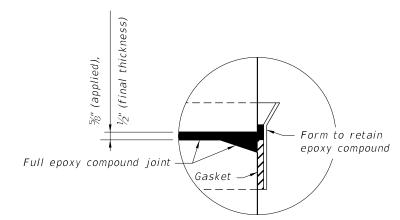
DETAIL SHOWING TYPICAL COVER

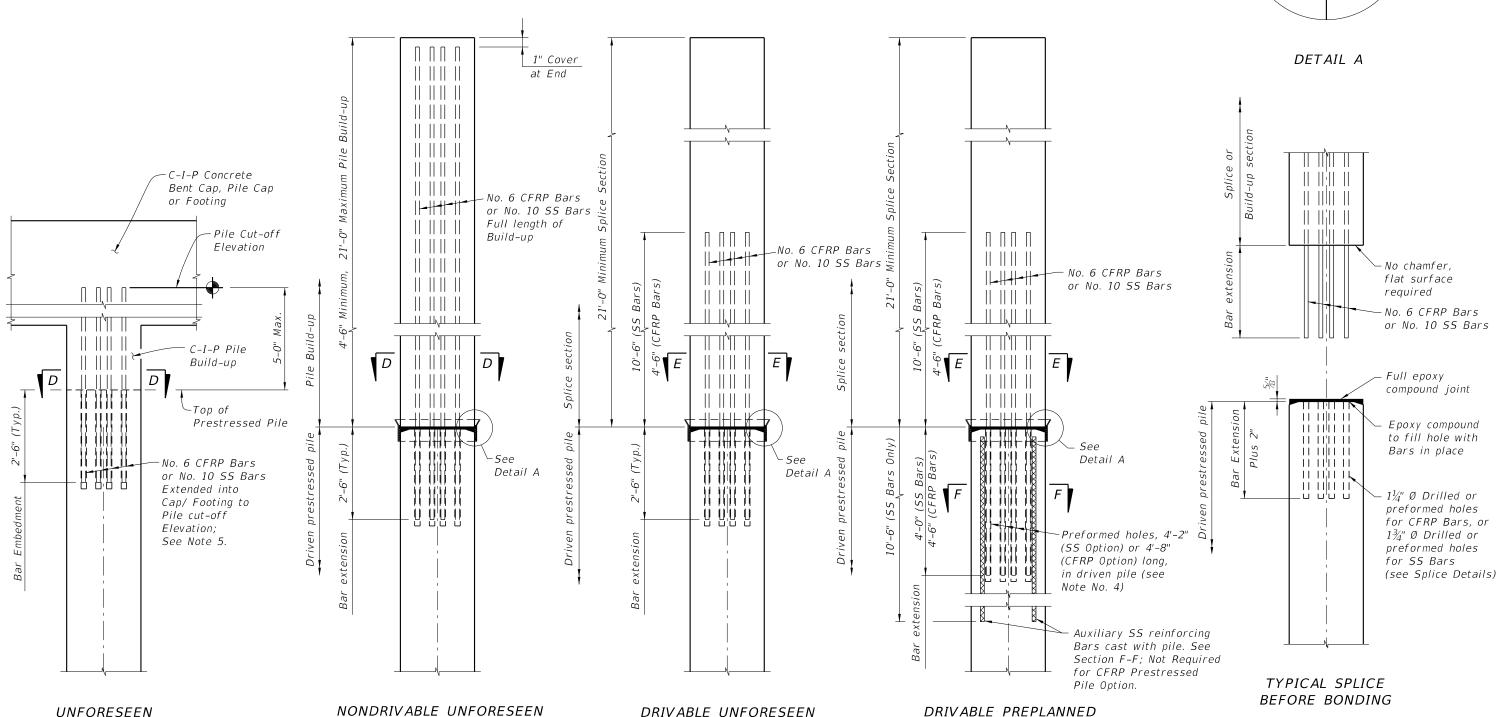
PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-102), the Prestressed Concrete Pile Standards (Index 455-112, 455-114, 455-118, 455-124, 455-130, and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - Piles: Class V Α.
 - See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required for options using stainless steel strand and reinforcing.
- 3. Concrete strength at time of prestress transfer:
- A. Piles: 4,000 psi minimum.
- 4. Reinforcing:
 - A. Bars:
 - a. Stainless Steel: Meet the requirements of Specification Section 931 for Type 304, Grade 75.
 - b. Carbon FRP: Meet the requirements of Specification Section 932.
 - Prestressing Strands:
 - a. Stainless Steel: Seven-wire HSSS, Grade 240
 - strand, meeting the requirements of Specification Section 933.
 - b. Carbon FRP: Meet the requirements of Specification Section 933.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
 - One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

NOTES:

- 1. For Sections D-D, & E-E, see Index 455-112, 455-114, 455-118, 455-124 or 455-130 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- 3. In cases where pile splices are desired due to length limitations in shipping and/or handling, the "Drivable Preplanned Prestressed Precast Splice Detail" shall be used.
- 4. When preformed dowel holes are utilized, the 1" spiral tie pitch shall be continued to 4'-0" below the head of the pile, See Index 455-118, 455-124. Preformed holes shall utilize either removable preforming material or stay-in-place corrugated galvanized steel ducts. Stay-in-place ducts shall be fabricated from galvanized sheet steel meeting the requirements of ASTM A653, Coating Designation G90, 26 gauge. Ducts shall be 1½" diameter for CFRP Bars, and 2" diameter for SS Bars with a minimum corrugation (rib) height of 0.12 in. Ducts shall be fabricated with either welded or interlocked seams. Galvanizing of welded seams will not be required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 6 CFRP Bars or No. 10 SS into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.





PRESTRESSED PRECAST

PILE SPLICE DETAIL

12:03:15

10/14/2023

LAST O DESCRIPTION:
REVISION S O DESCRIPTION:

REINFORCED C-I-P

PILE BUILD-UP DETAIL



REINFORCED PRECAST

PILE BUILD-UP DETAIL

FY 2024-25 STANDARD PLANS

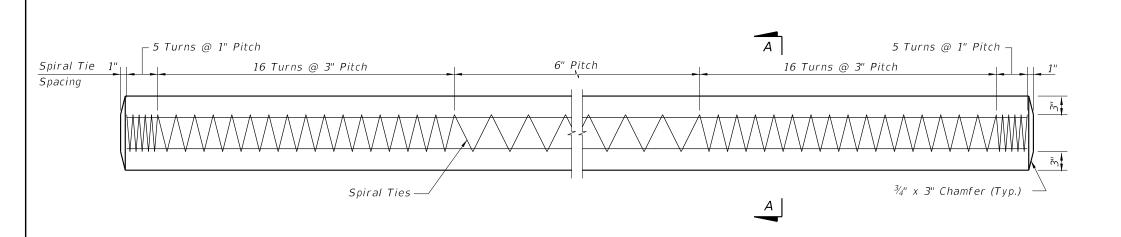
SQUARE CFRP & SS PRESTRESSED CONCRETE PILE SPLICES

PRESTRESSED PRECAST

PILE SPLICE DETAIL

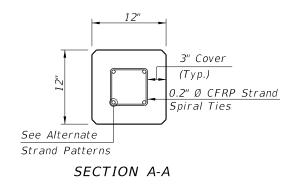
INDEX

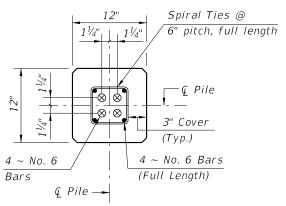
sнеет **1 of 1**



ALTERNATE STRAND PATTERNS

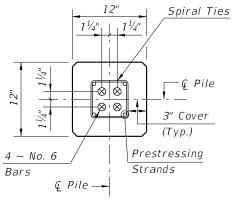
 $4 \sim 0.6$ " Ø, CFRP 7-Strand, at 42 kips $4 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 41 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

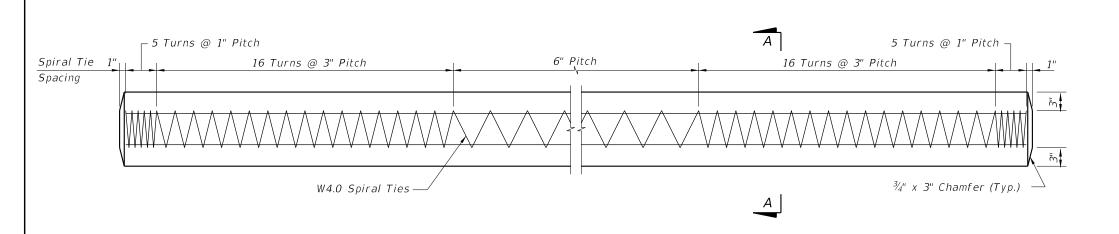
(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

NOTES.

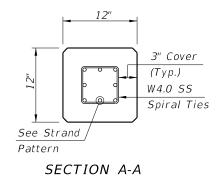
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized.

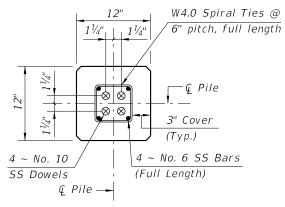
CFRP PRESTRESSED PILE DETAILS



STRAND PATTERN

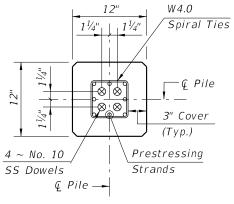
 $8 \sim \frac{1}{2}$ " Ø, HSSS at 24 kips $8 \sim 0.6$ " Ø, HSSS at 26 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

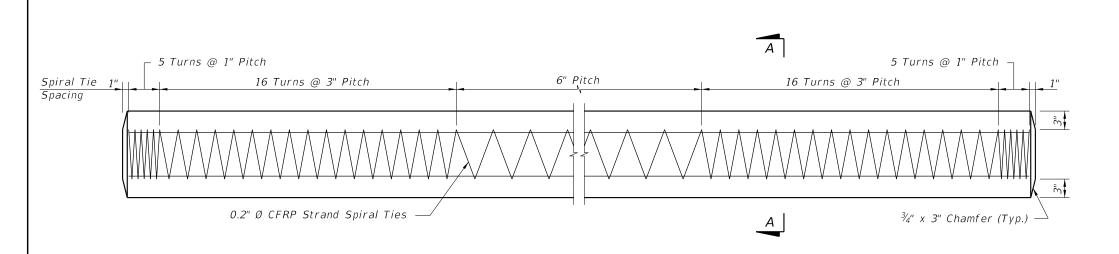
(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

NOTES

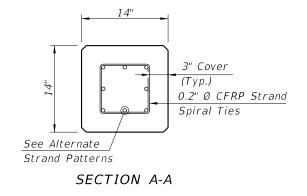
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized.
 The strands shall be located as follows:
 Place one strand at each corner and place the remaining strands equally spaced between the corner strands.
 The total strand pattern shall be concentric with the nominal concrete section of the pile.

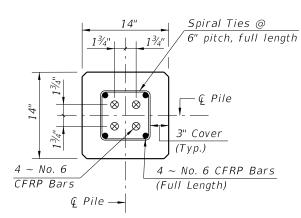
SS PRESTRESSED PILE DETAILS



ALTERNATE STRAND PATTERNS

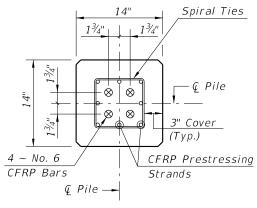
8 ~ 0.6" Ø, CFRP 7-Strand, at 31.5 kips $8 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 30.5 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and equally space the remaining strands between the corner strands. The total strand pattern shall be concentric with the nominal

concrete section of the pile.

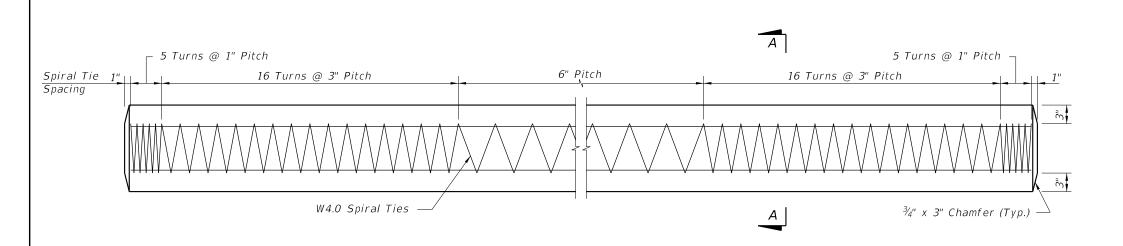
CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

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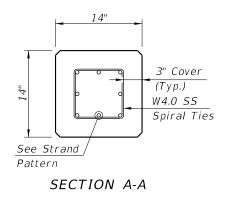
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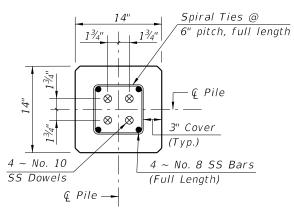
1 of 2



STRAND PATTERN

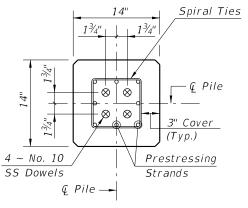
12 ~ ½" Ø, HSSS at 23 kips 8 ~ 0.6" Ø, HSSS at 35 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

FDOT

FY 2024-25 STANDARD PLANS

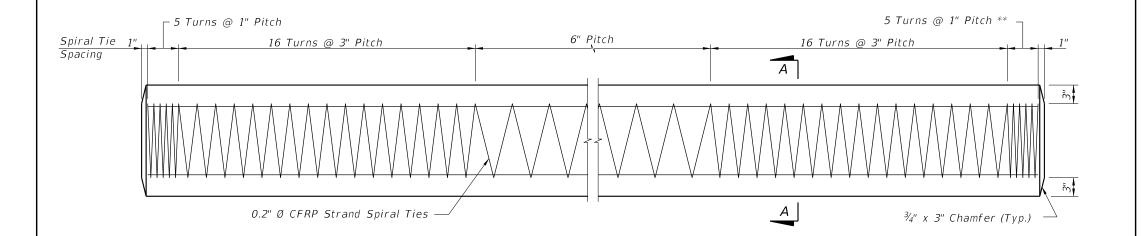
14" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

SHEET

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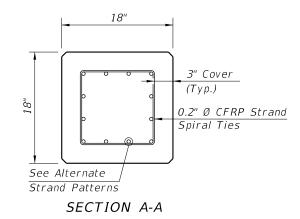
455-114 2 of 2



ELEVATION

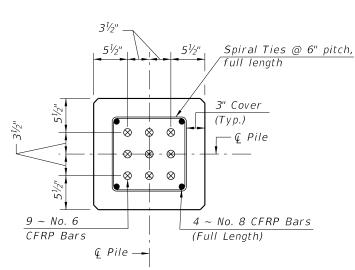
ALTERNATE STRAND PATTERNS

 $12 \sim 0.6$ " Ø, CFRP 7-Strand, at 34 kips $12 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 33 kips



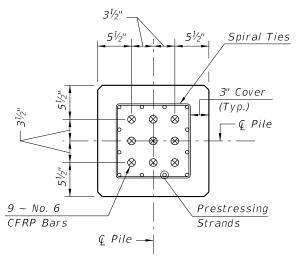
NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized.
 The strands shall be located as follows:
 Place one strand at each corner and place the remaining strands equally spaced between the corner strands.
 The total strand pattern shall be concentric with the nominal concrete section of the pile.



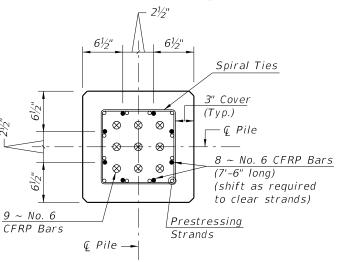
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

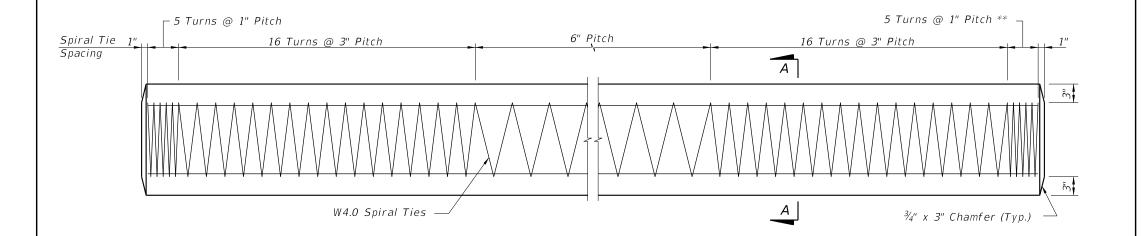
(See Drivable Preplanned Prestressed Precast Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

10/14/2023 12

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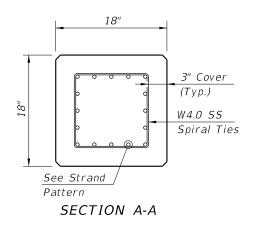
455-118 1 of 2



ELEVATION

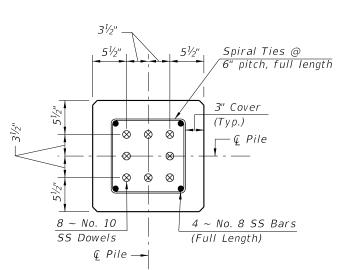
STRAND PATTERN

 $16 \sim \frac{1}{2}$ " Ø, HSSS, at 26 kips 12 ~ 0.6" Ø, HSSS, at 35 kips



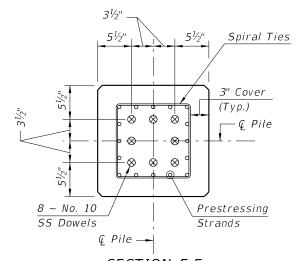
NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



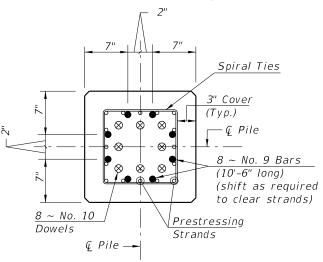
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

REVISION 11/01/20

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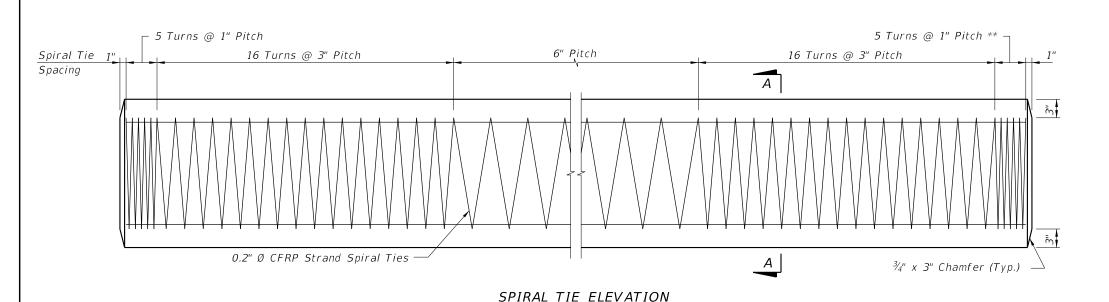
FY 2024-25 STANDARD PLANS

18" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

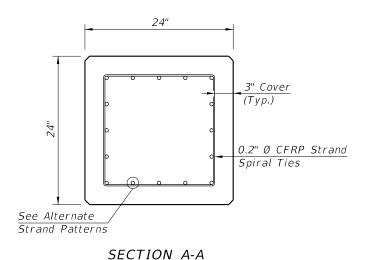
SHEET 2 of 2

455-118



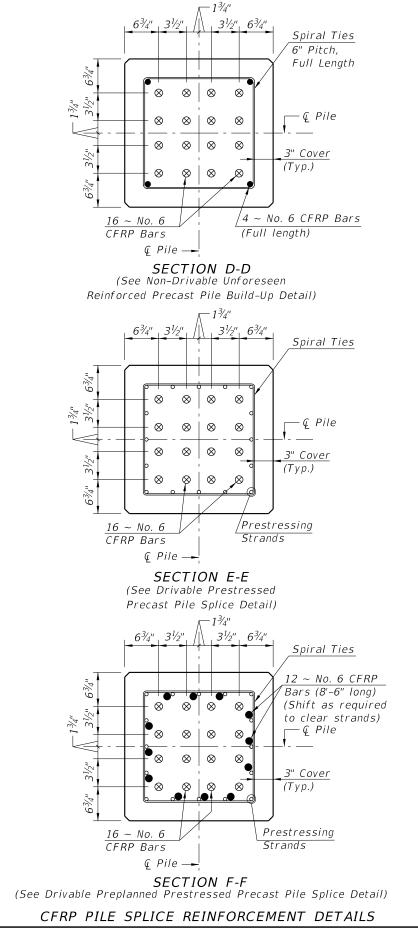
ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips $20 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 35 kips



NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



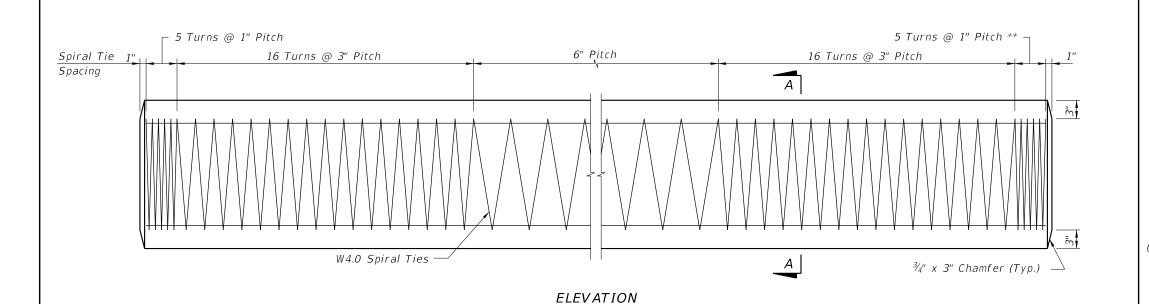
CFRP PRESTRESSED PILE DETAILS

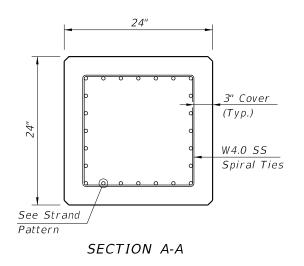
REVISION 11/01/20

DESCRIPTION:

FDOT

FY 2024-25 STANDARD PLANS



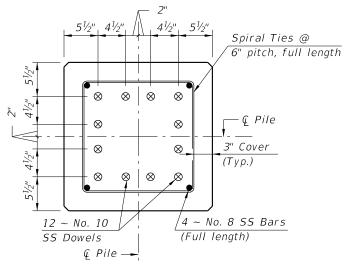


STRAND PATTERN

 $28 \sim \frac{1}{2}$ " Ø, HSSS at 26 kips 20 ~ 0.6" Ø, HSSS at 35 kips

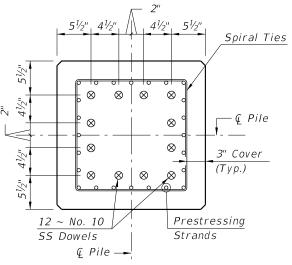
NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



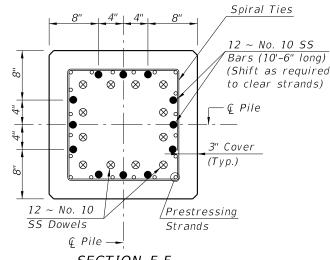
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

DESCRIPTION:

FDOT

FY 2024-25 STANDARD PLANS

24" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

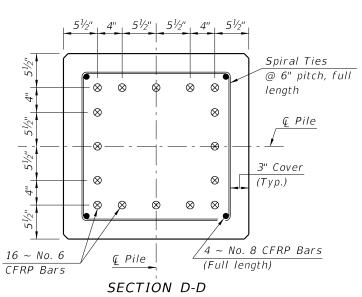
SHEET

455-124

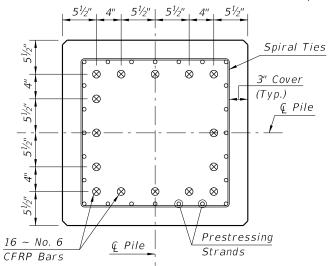
20 ~ 0.6" Ø, CFRP 7-Strand at 38 kips 20 ~ ½" Ø, CFRP Single-Strand at 37 kips

DESCRIPTION:

- Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.
- 2. CONTRACTOR OPTION: The 30" pile may be cast SOLID by omitting the 18" Ø void. In this event, the Contractor shall submit calculations for approval and a proposed strand configuration that provide net prestressing after losses equal to 1000 psi. Alternate configurations for the Diagonal Ties, to maintain the position of the 4 ~ #6 Bars, may be approved by the Engineer.
- 3. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.

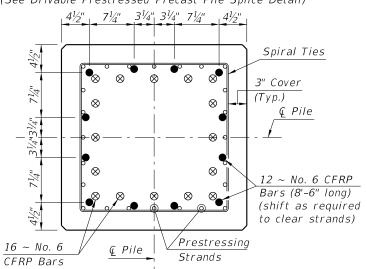


(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE DETAILS

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

FDOT

FY 2024-25 STANDARD PLANS

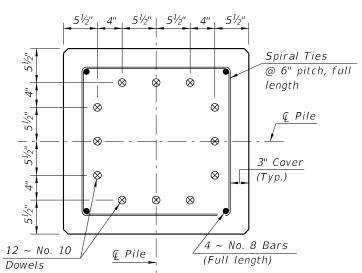
CONCRETE PILE

INDEX 455-130 SHEET

24 ~ 0.6" Ø, HSSS at 35 kips

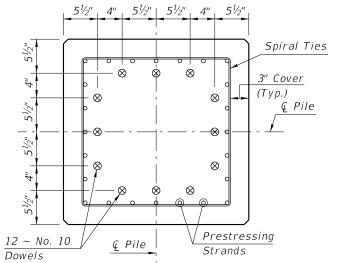
DESCRIPTION:

- 2. CONTRACTOR OPTION: The 30" pile may be cast SOLID by omitting the 18" Ø void. In this event, the Contractor shall submit calculations for approval and a proposed strand configuration that provide net prestressing after losses equal to 1000 psi. Alternate configurations for the Diagonal Ties, to maintain the position of the 4 ~ #8 Bars, may be approved by the Engineer.
- 3. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.



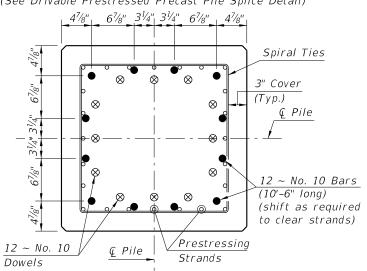
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

SS PILE SPLICE DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

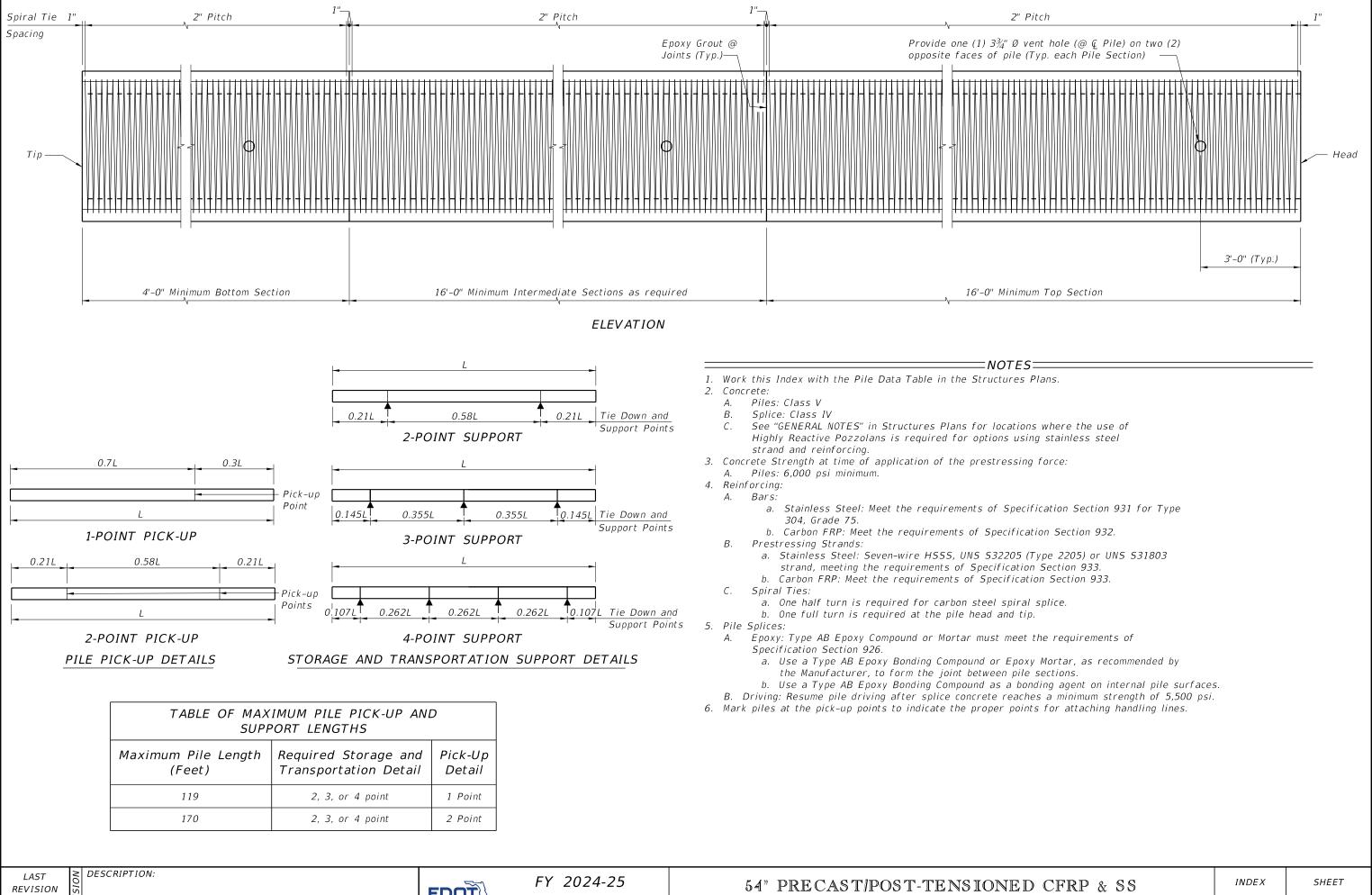
FDOT

FY 2024-25 STANDARD PLANS

30" SQUARE CFRP & SS PRESTRESSED

INDEX

SHEET



11/01/23

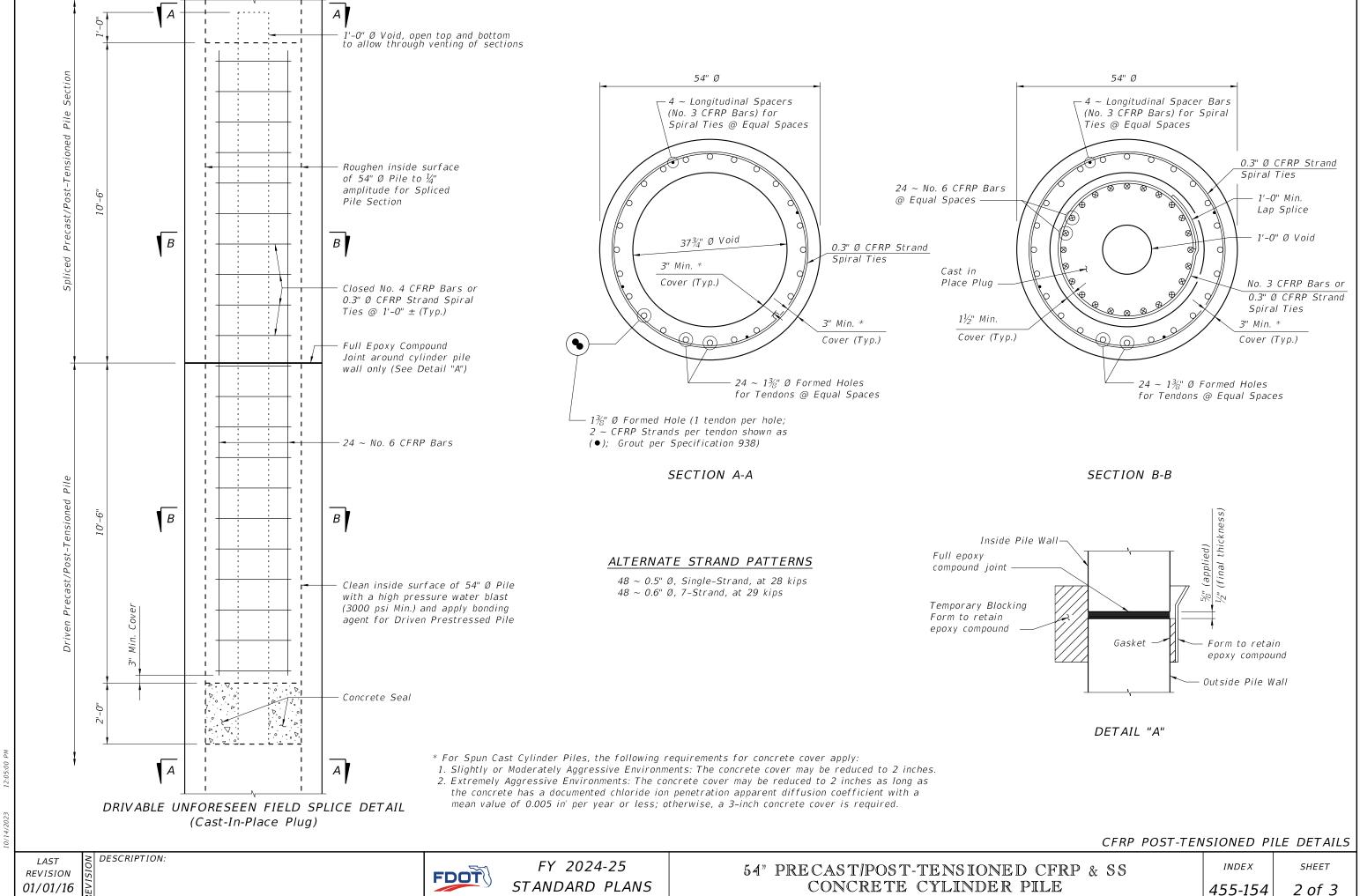
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STANDARD PLANS

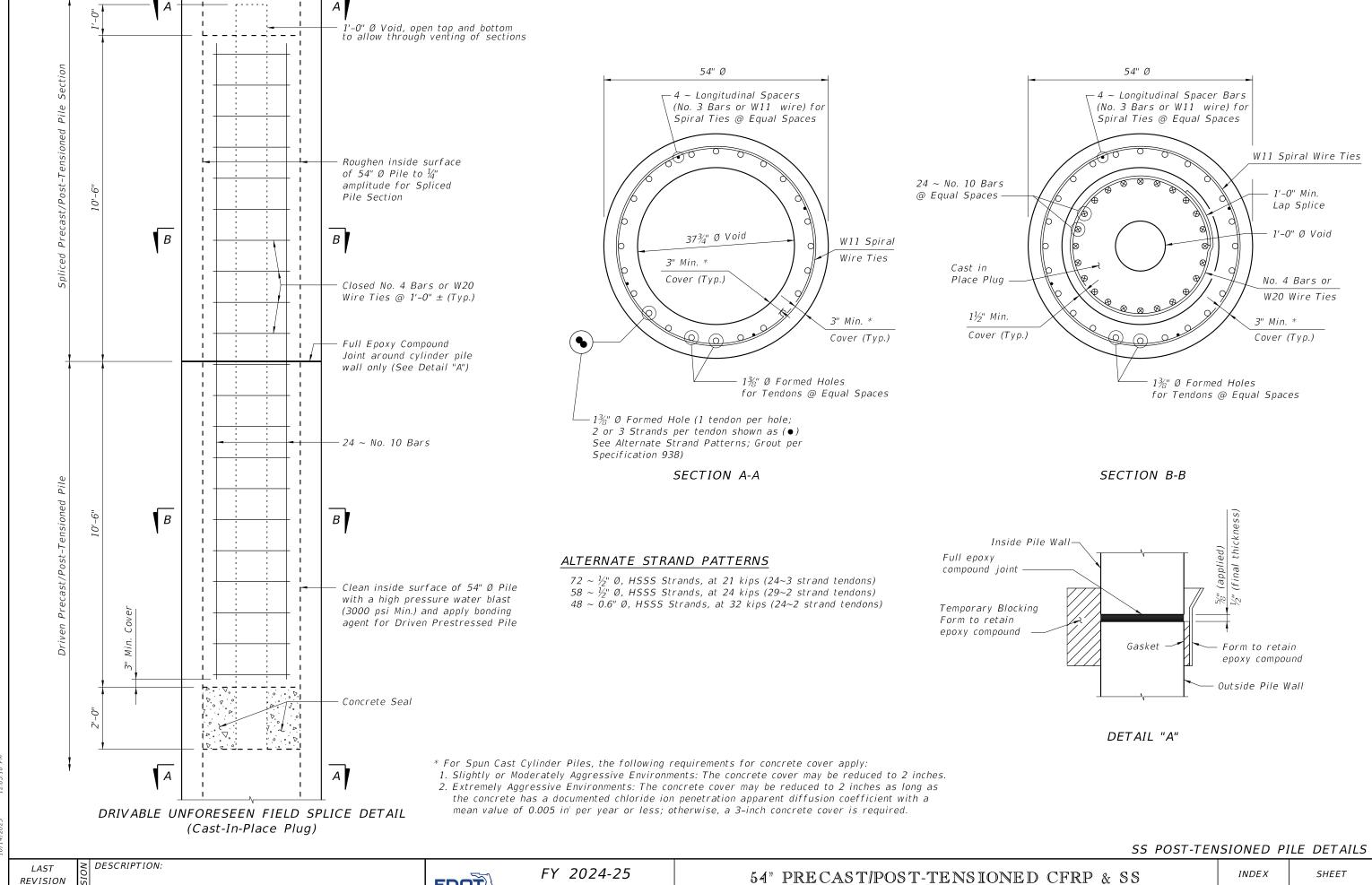
CONCRETE CYLINDER PILE

455-154

1 of 3



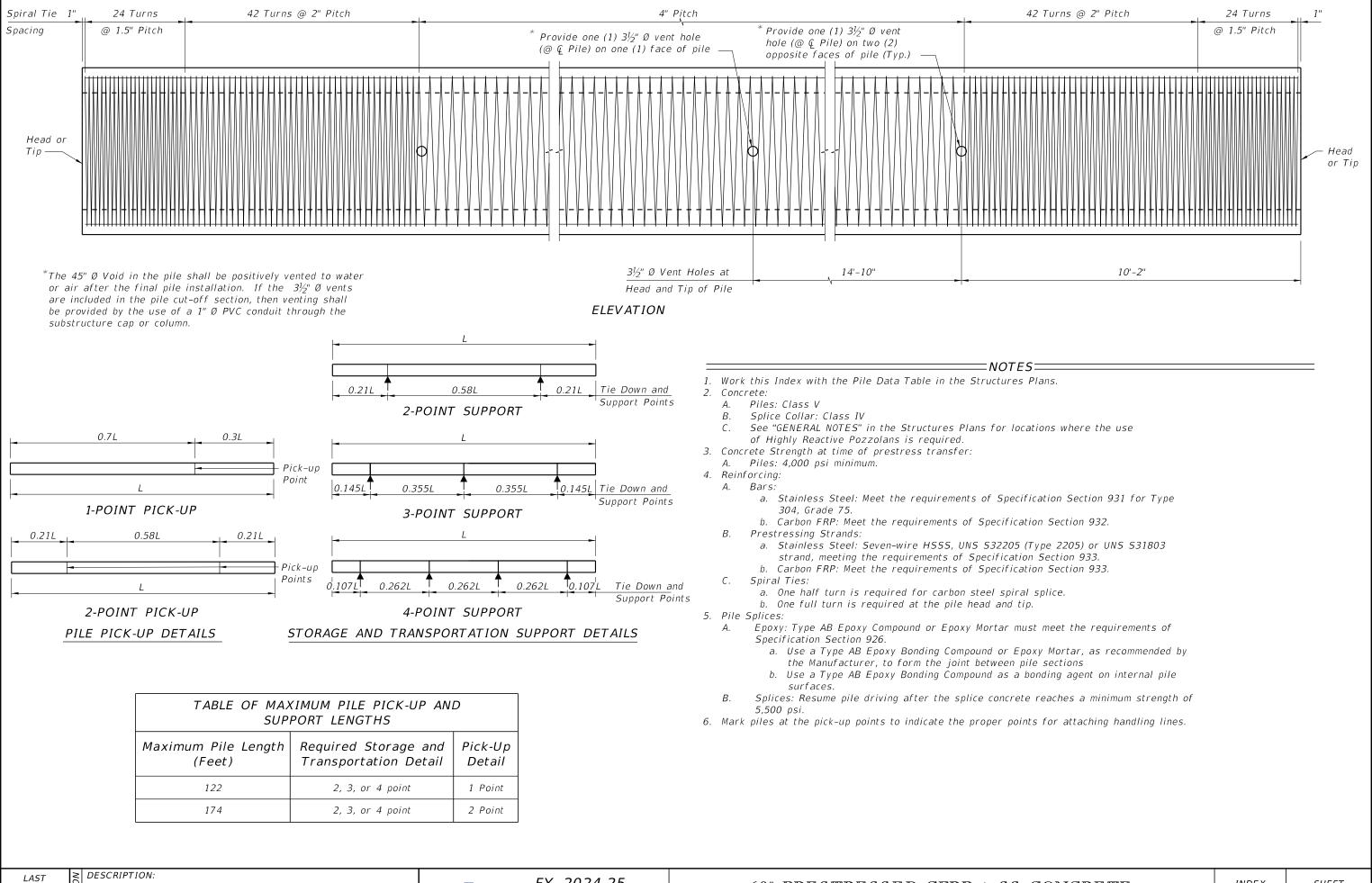
2 of 3



01/01/16

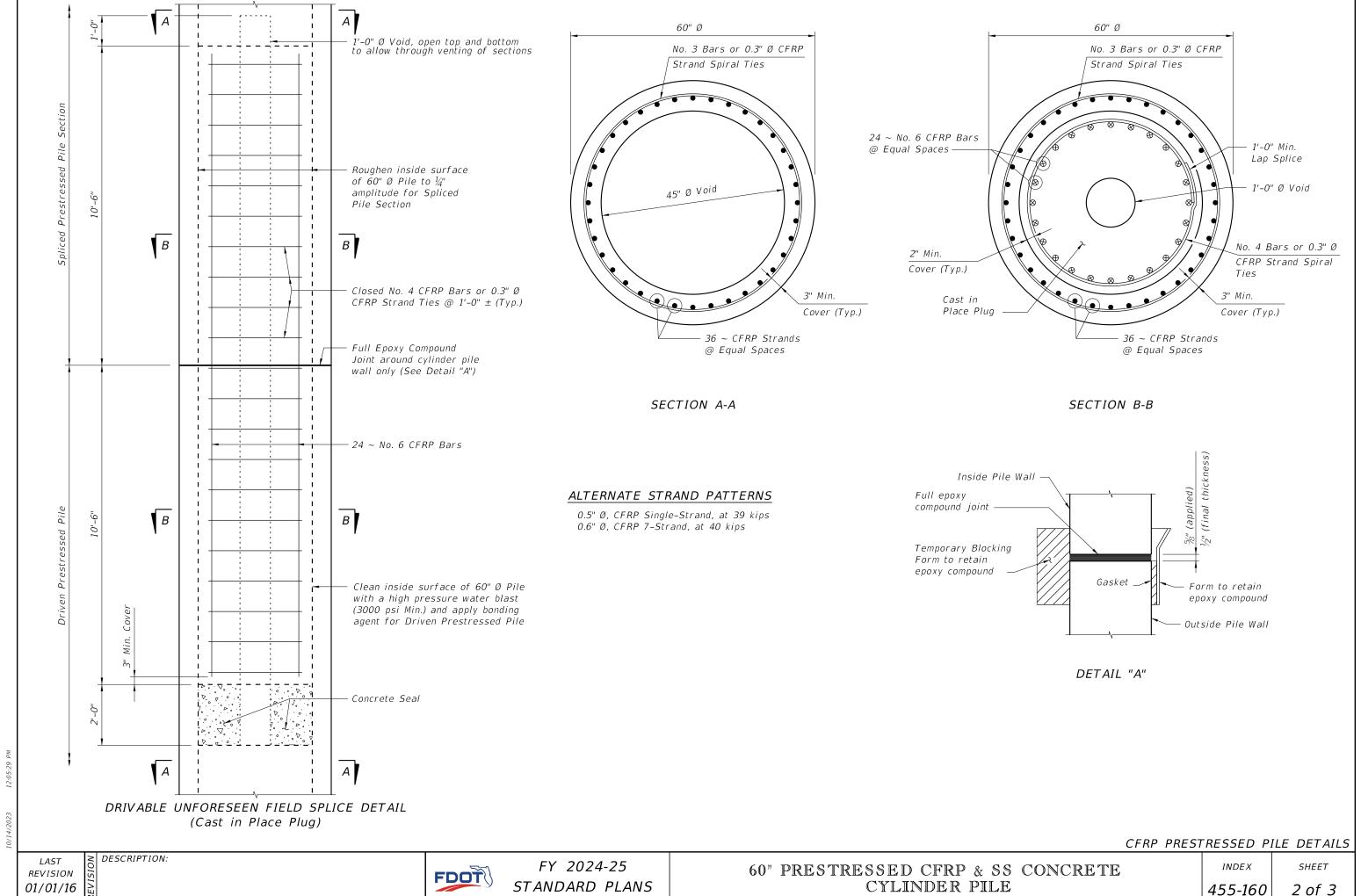
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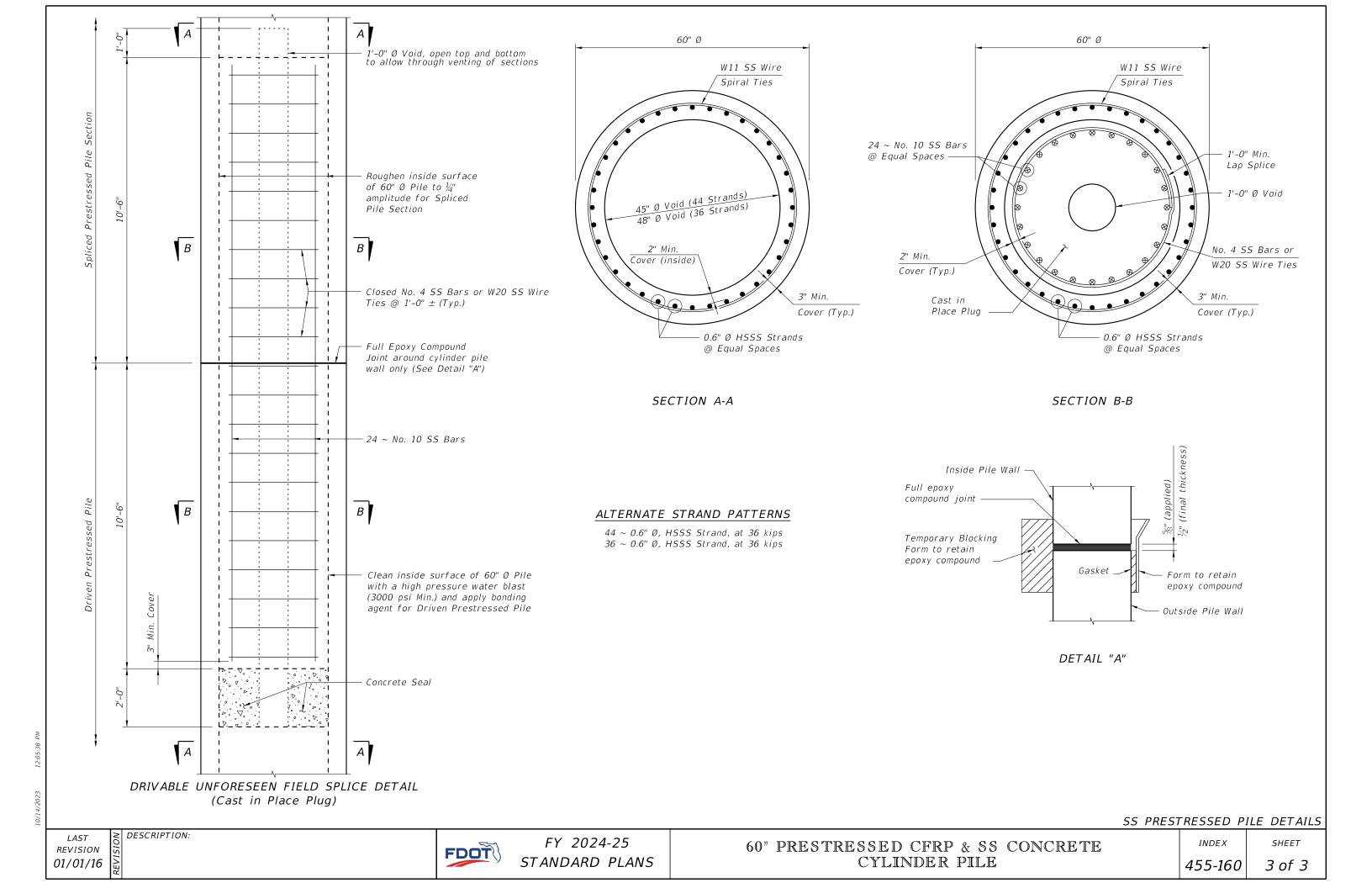
STANDARD PLANS

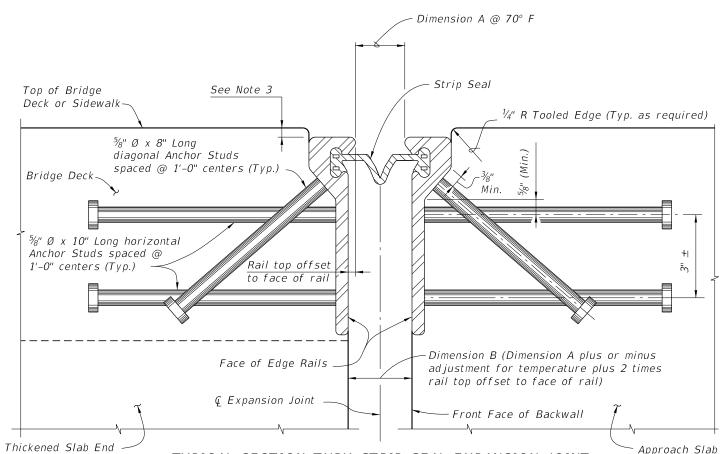


LAST REVISION 11/01/22

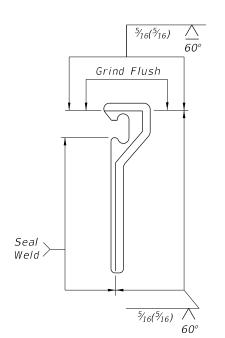
FDOT



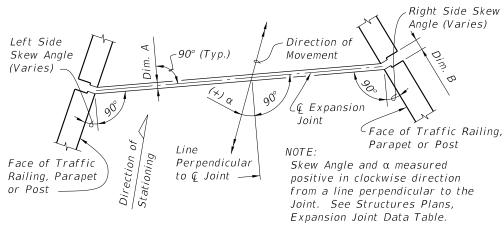




TYPICAL SECTION THRU STRIP SEAL EXPANSION JOINT (Begin or End Concrete Girder Bridge shown, Intermediate Supports and Steel Girder Bridge similar. Reinforcing Steel and Girder details not shown for clarity.)



SHOP SPLICE DETAIL



MOVEMENT SCHEMATIC

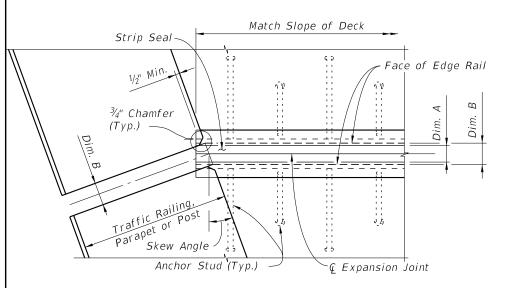
GENERAL NOTES:

- 1. Furnish Strip Seal Expansion Joint Systems in accordance with Specification Section 458.
- 2. Shape of Edge Rail shown is representative, minor variations depending on manufacturer are permitted.
- 3. Recess the Edge Rail below the concrete surface in accordance with Specification Section 458.
- 4. Refer to the Expansion Joint Data Table in the Structures Plans for joint movement and Dimension A.
- 5. Refer to Specification Section 458 for installation and fabrication requirements.

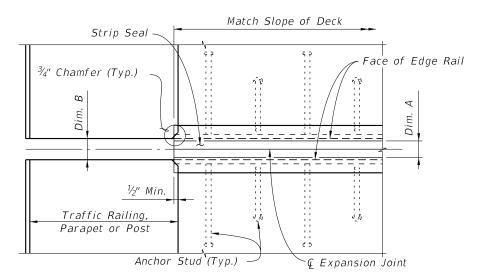
REVISION 11/01/19

DESCRIPTION:

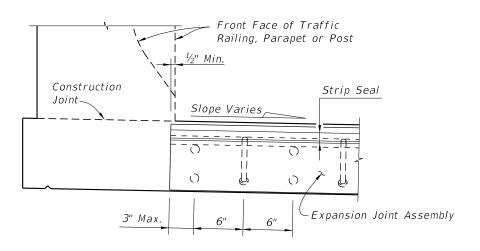
FDOT



PARTIAL PLAN VIEW OF SKEWED JOINTS



PARTIAL PLAN VIEW OF NONSKEWED JOINTS



PARTIAL SECTION ALONG Q JOINT

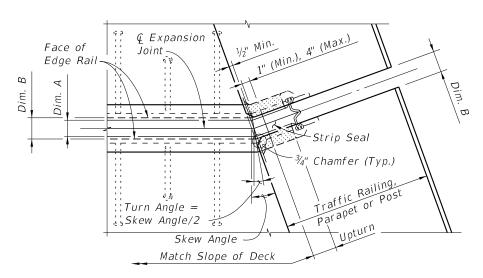
REVISION 11/01/19

JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPE ≥ 1%

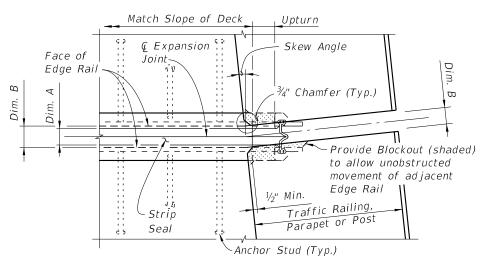
(Sidewalk Cover Plate where applicable not shown for clarity)

FDOT

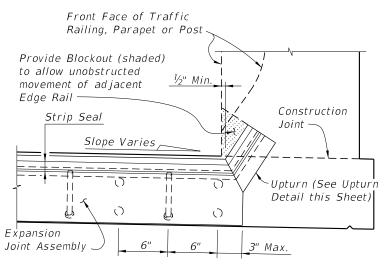
FY 2024-25 STANDARD PLANS



PARTIAL PLAN VIEW OF JOINTS SKEWED GREATER THAN 6°

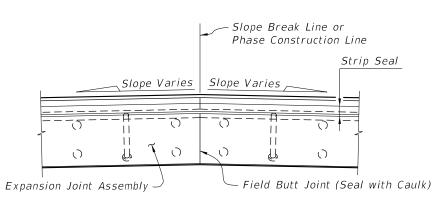


PARTIAL PLAN VIEW OF NONSKEWED JOINTS & JOINTS SKEWED 6° OR LESS

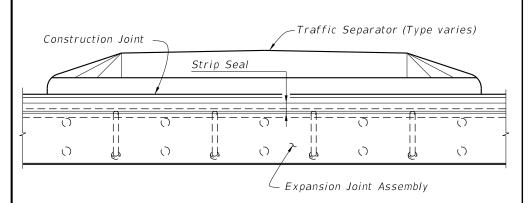


PARTIAL SECTION ALONG G JOINT

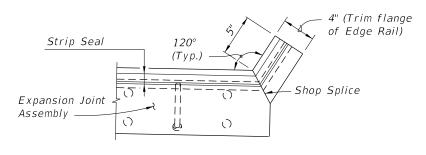
JOINT TREATMENT AT LOW SIDE OF DECK & HIGH SIDE OF DECK WITH SLOPE < 1% (Sidewalk Cover Plate where applicable not shown for clarity)



PARTIAL SECTION ALONG Q JOINT AT FIELD BUTT JOINT LOCATION (CROWNED DECK OR SLAB SHOWN)



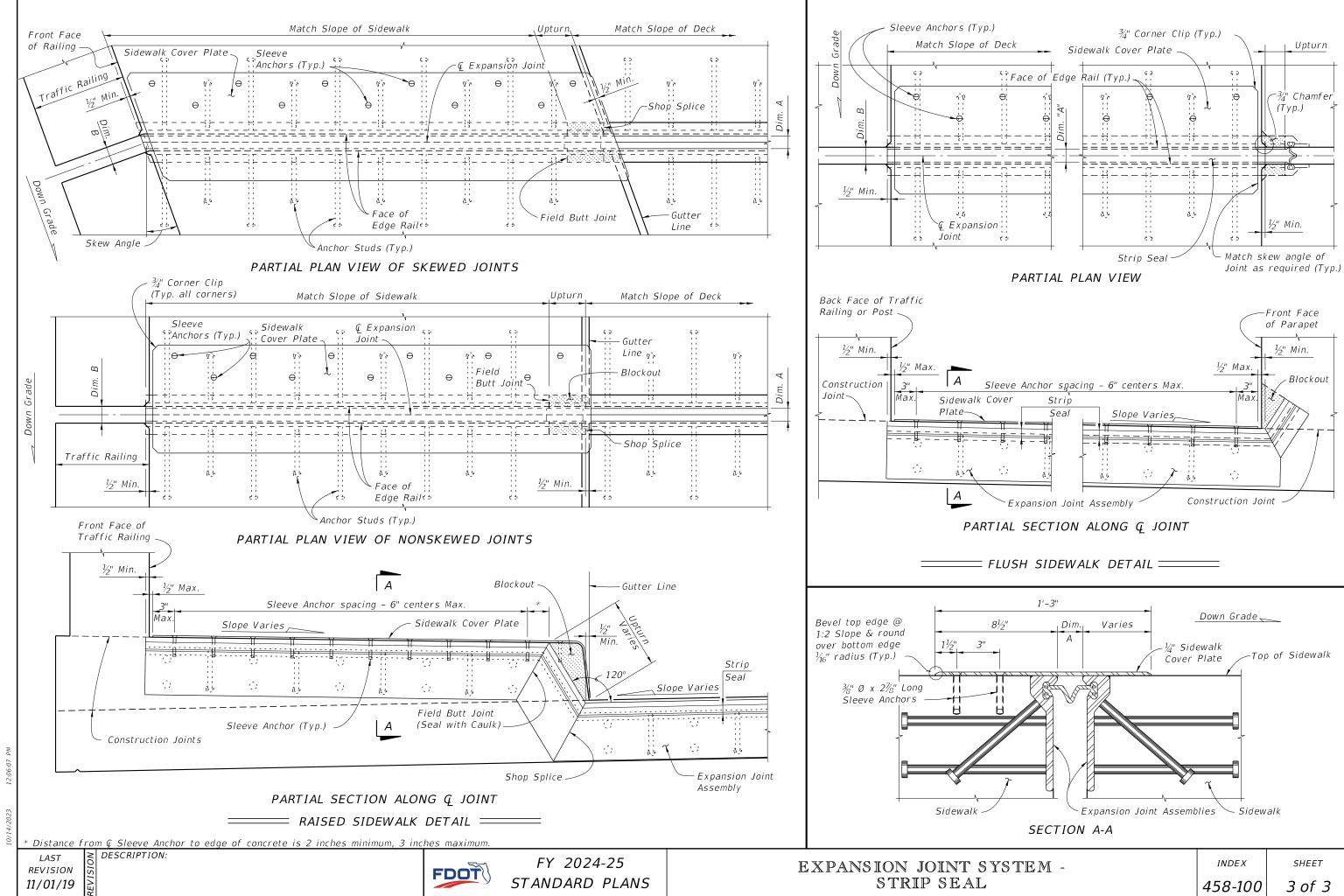
PARTIAL SECTION ALONG Q JOINT THRU TRAFFIC SEPARATOR

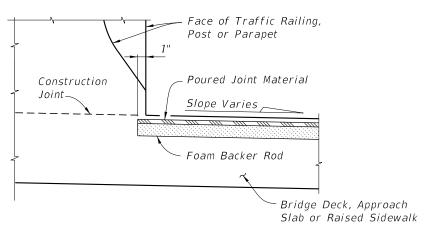


UPTURN DETAIL (TYPICAL AT TRAFFIC BARRIERS AND PARAPETS)

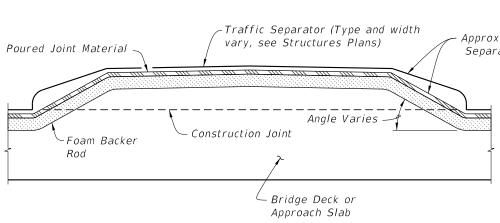
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SHEET

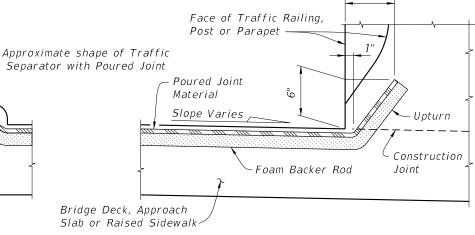




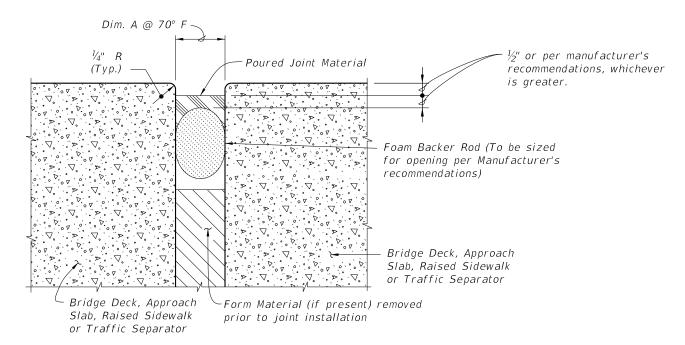
PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPES 1% OR GREATER



PARTIAL SECTION ALONG Q JOINT, JOINT TREATMENT AT TRAFFIC SEPARATOR



PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT LOW SIDE OF DECK OR HIGH SIDE OF DECK WITH SLOPES < 1%



TYPICAL SECTION THRU JOINT

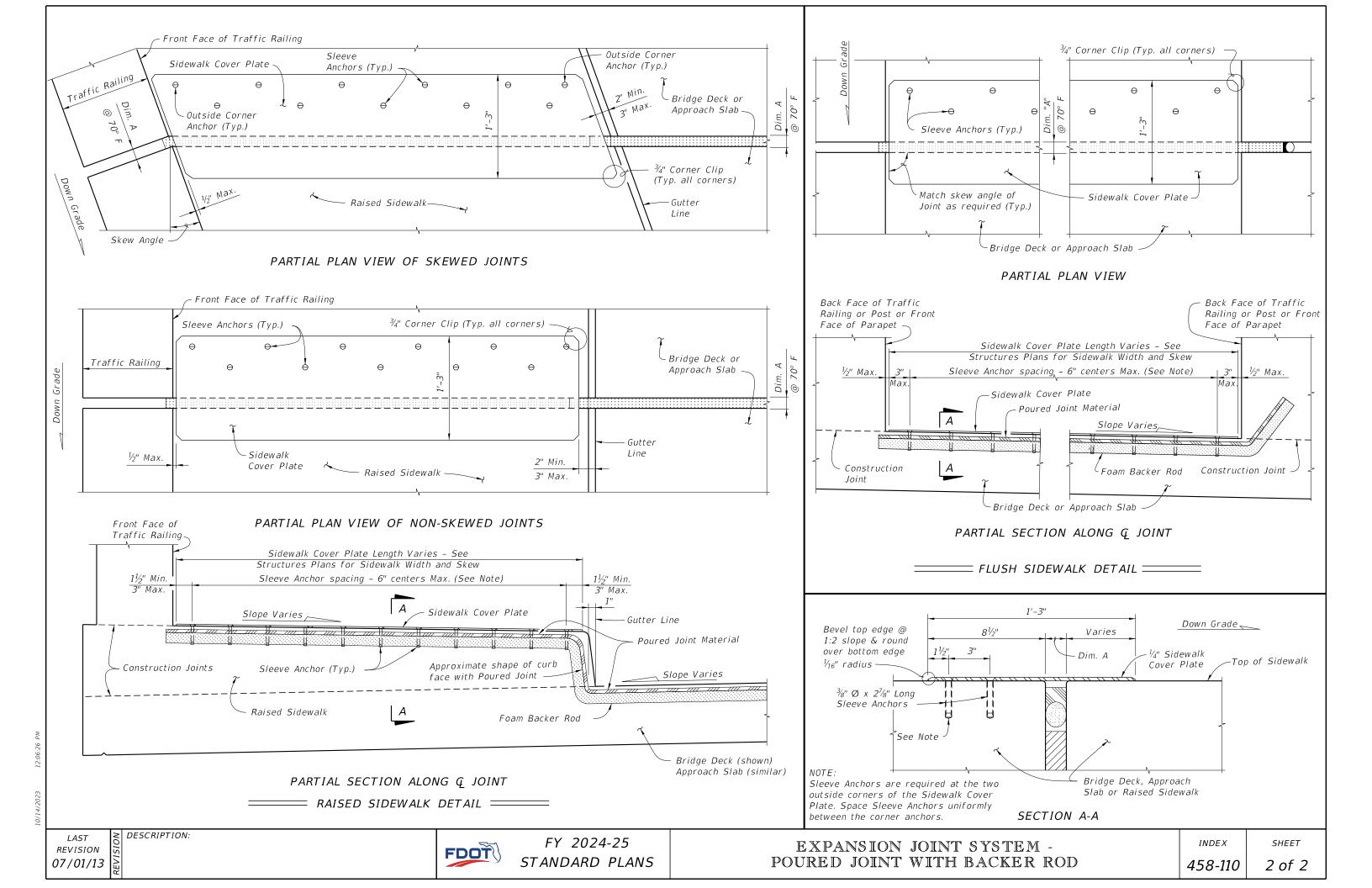
GENERAL NOTES:

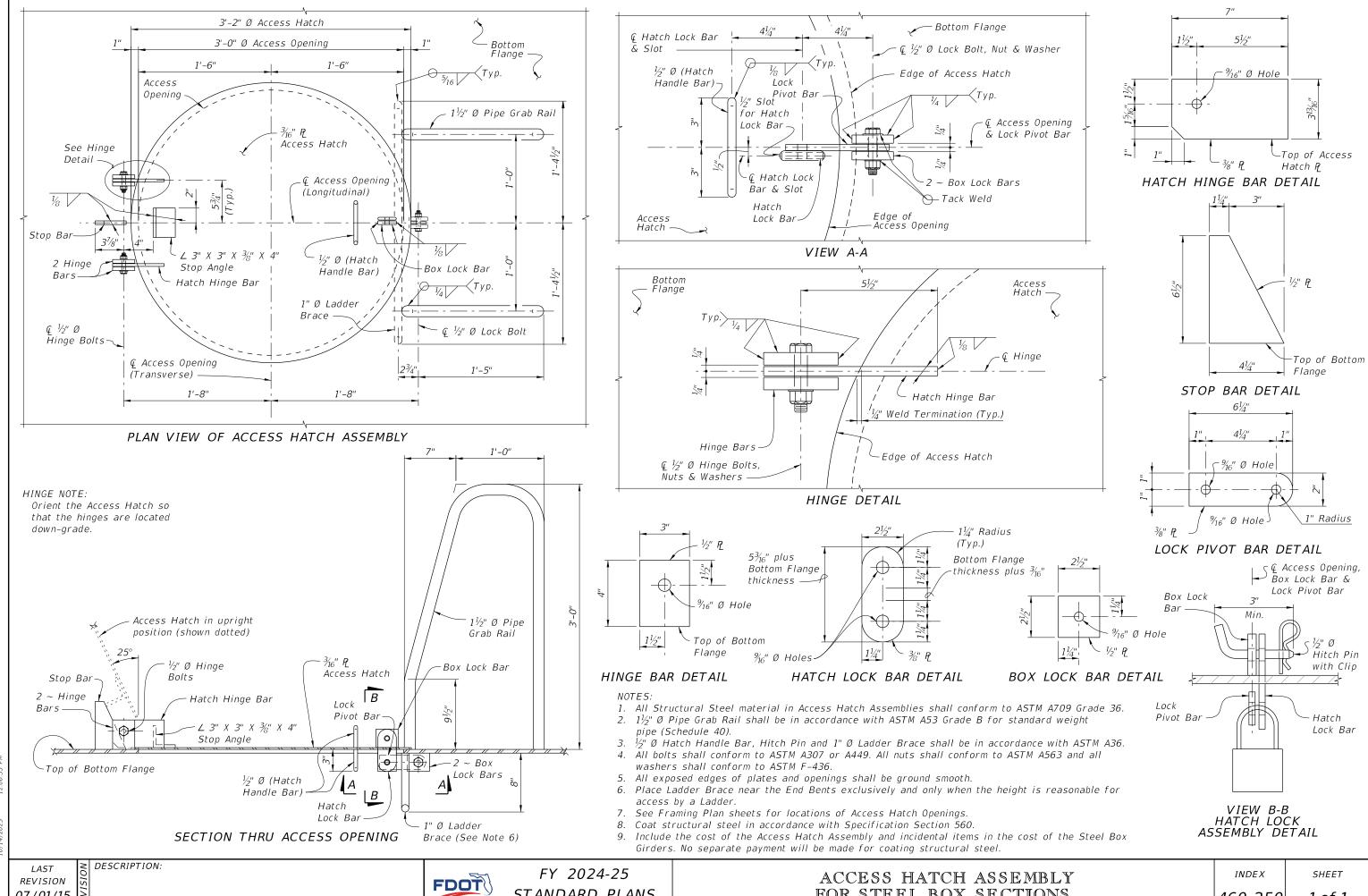
- 1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
- 2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.

REVISION 11/01/23

DESCRIPTION:



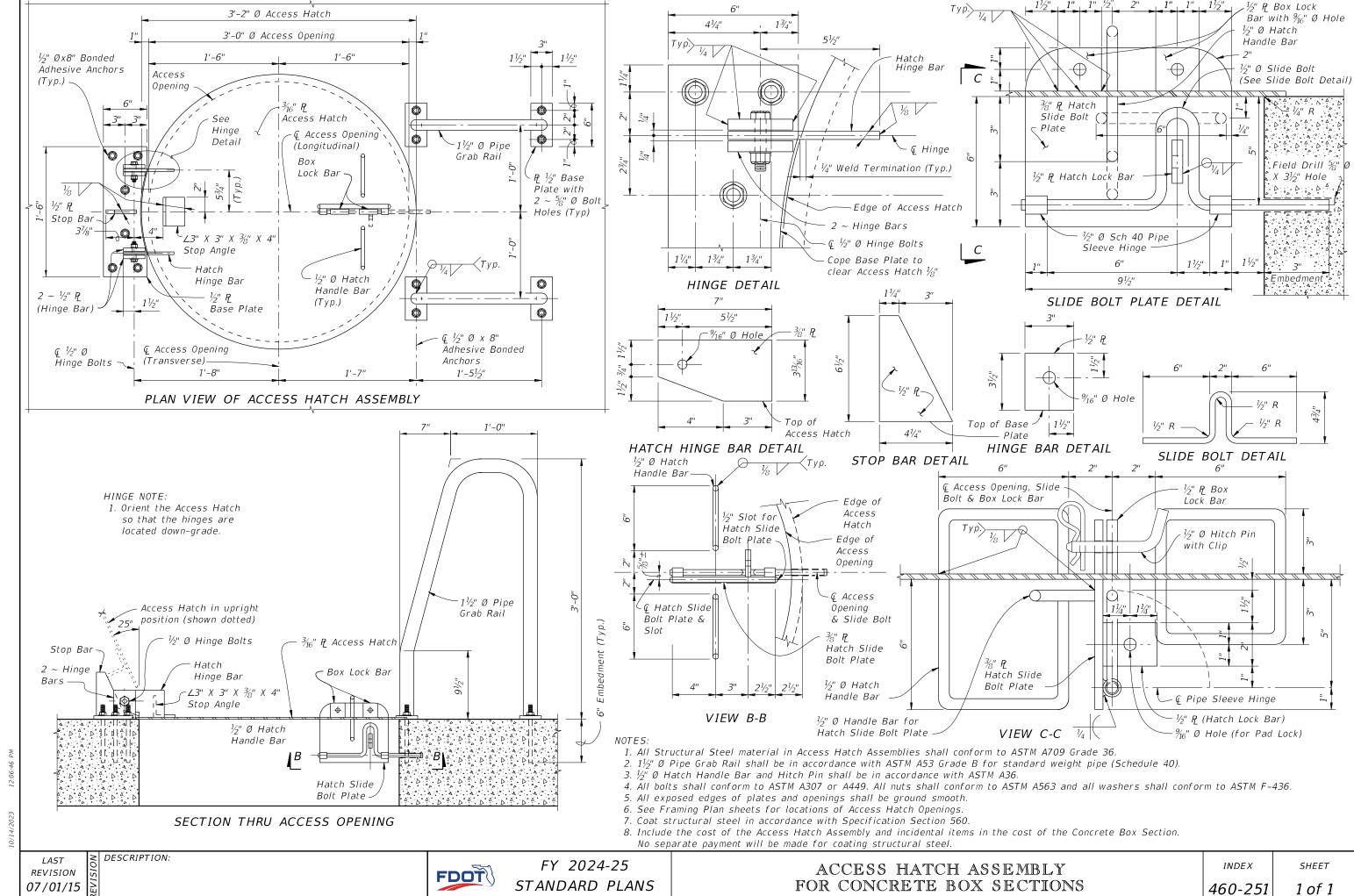


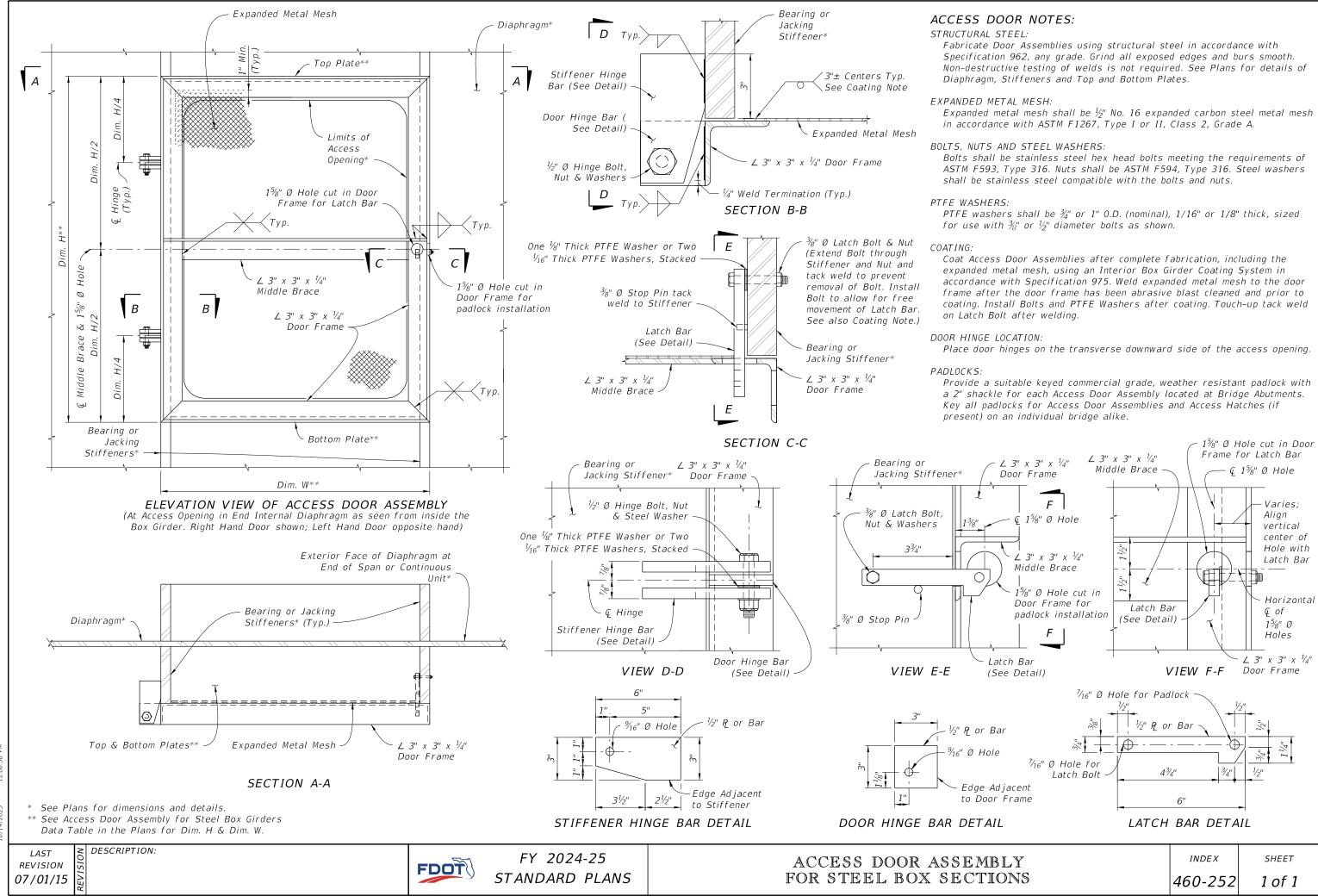


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10/14/20

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be 34" by 21/4" slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM A449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for $\frac{1}{8}$ " Ø anchor bolts; 55,000 lbs. for the $\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the $\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1½" spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

BEARING PADS: Provide plain Neoprene pads with a durometer hardness of 60 or 70 and meeting the requirements of Specification Section 932, for ancillary structures.

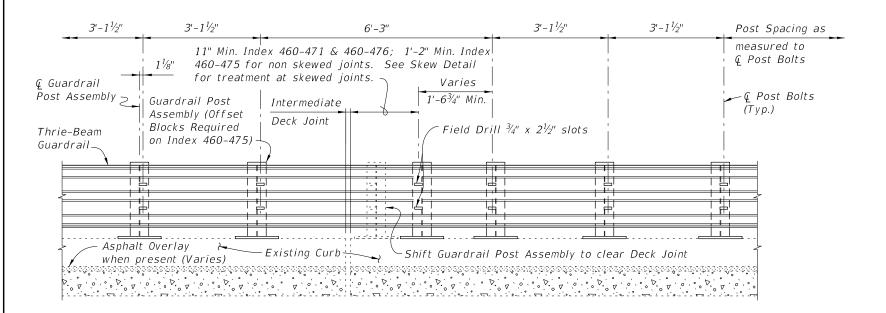
ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

BARRIER DELINEATORS: Install Barrier Delineators at the top of the guardrail offset blocks in accordance with Specification Section 705. Match the Barrier Delineators color (white or yellow) to the near edgeline.

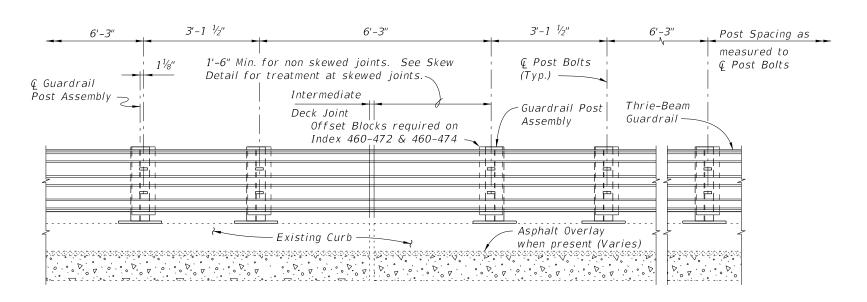
PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index 536-001 for details.

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

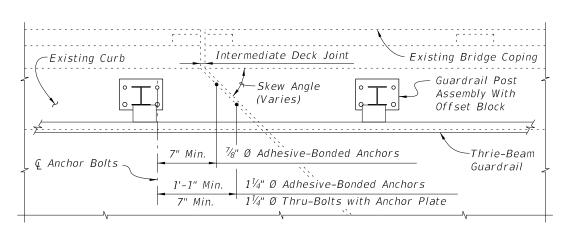
PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.



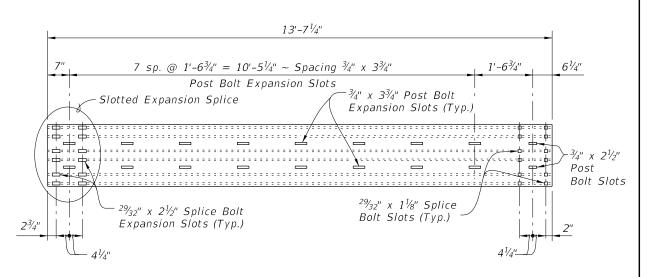
PARTIAL ELEVATION OF INSIDE FACE OF RAILING MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX 460-471, 460-475 & 460-476



PARTIAL ELEVATION OF INSIDE FACE OF RAILING MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX 460-472, 460-473 & 460-474

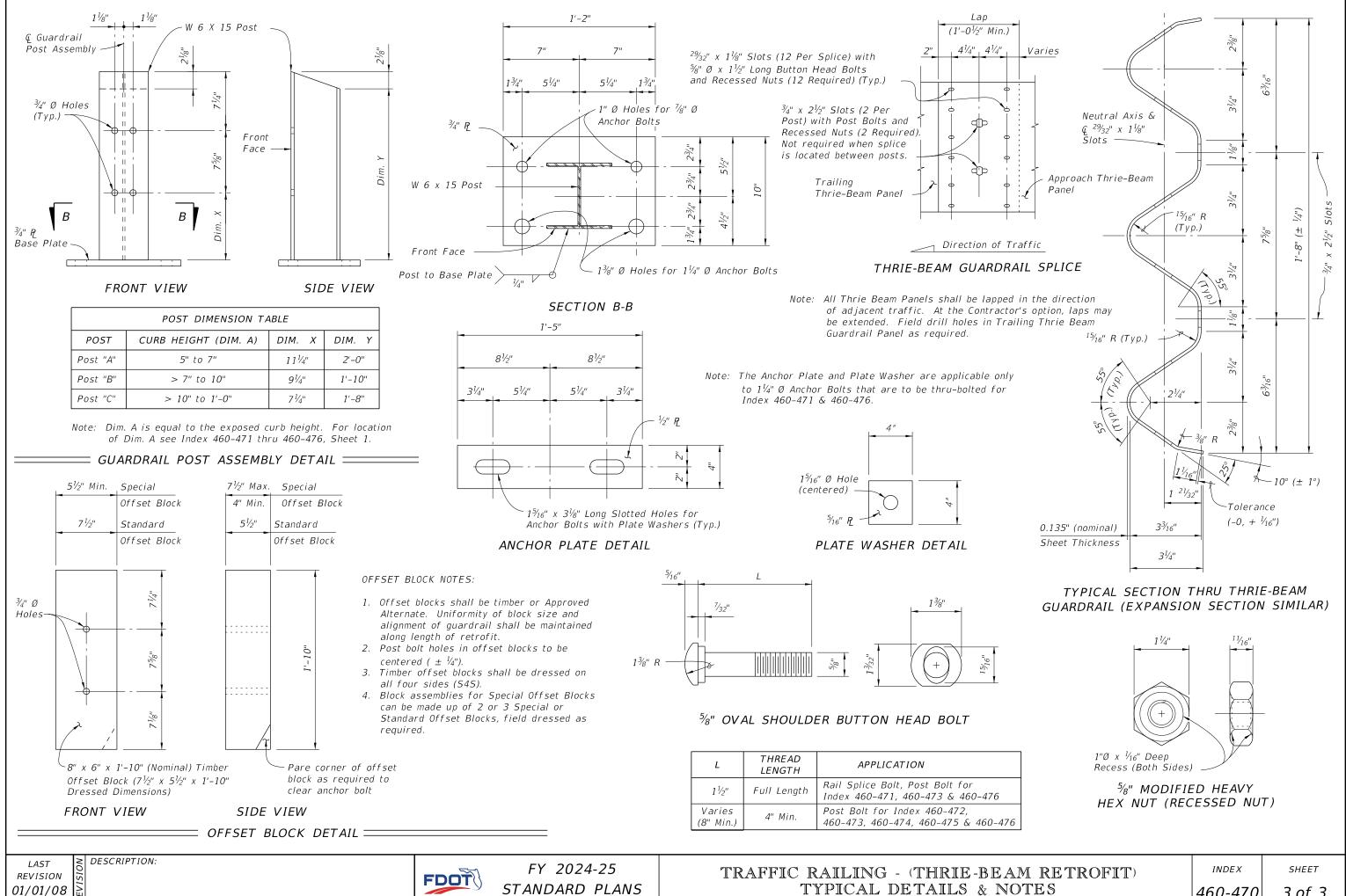


PARTIAL PLAN INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION

DESCRIPTION:



STANDARD PLANS

TYPICAL DETAILS & NOTES

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TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

REVISION 01/01/08

DESCRIPTION:

FDOT

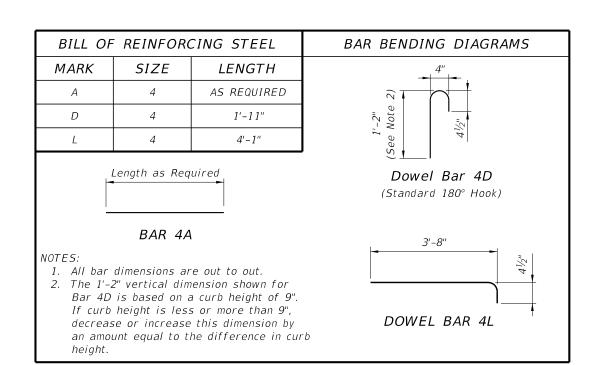
FY 2024-25 STANDARD PLANS

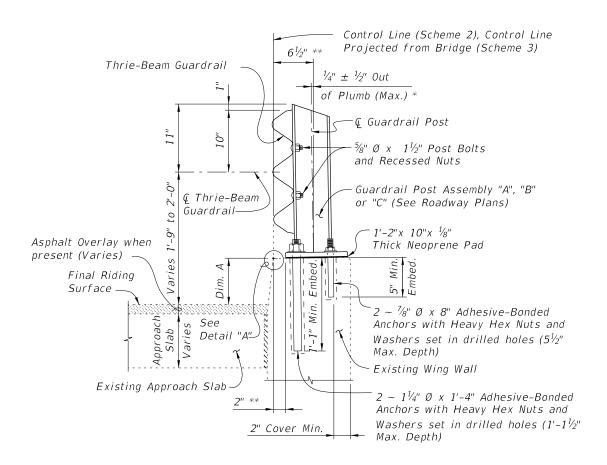
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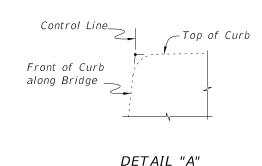
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

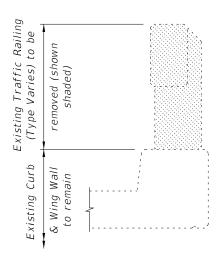




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

REVISION 01/01/08

DESCRIPTION:

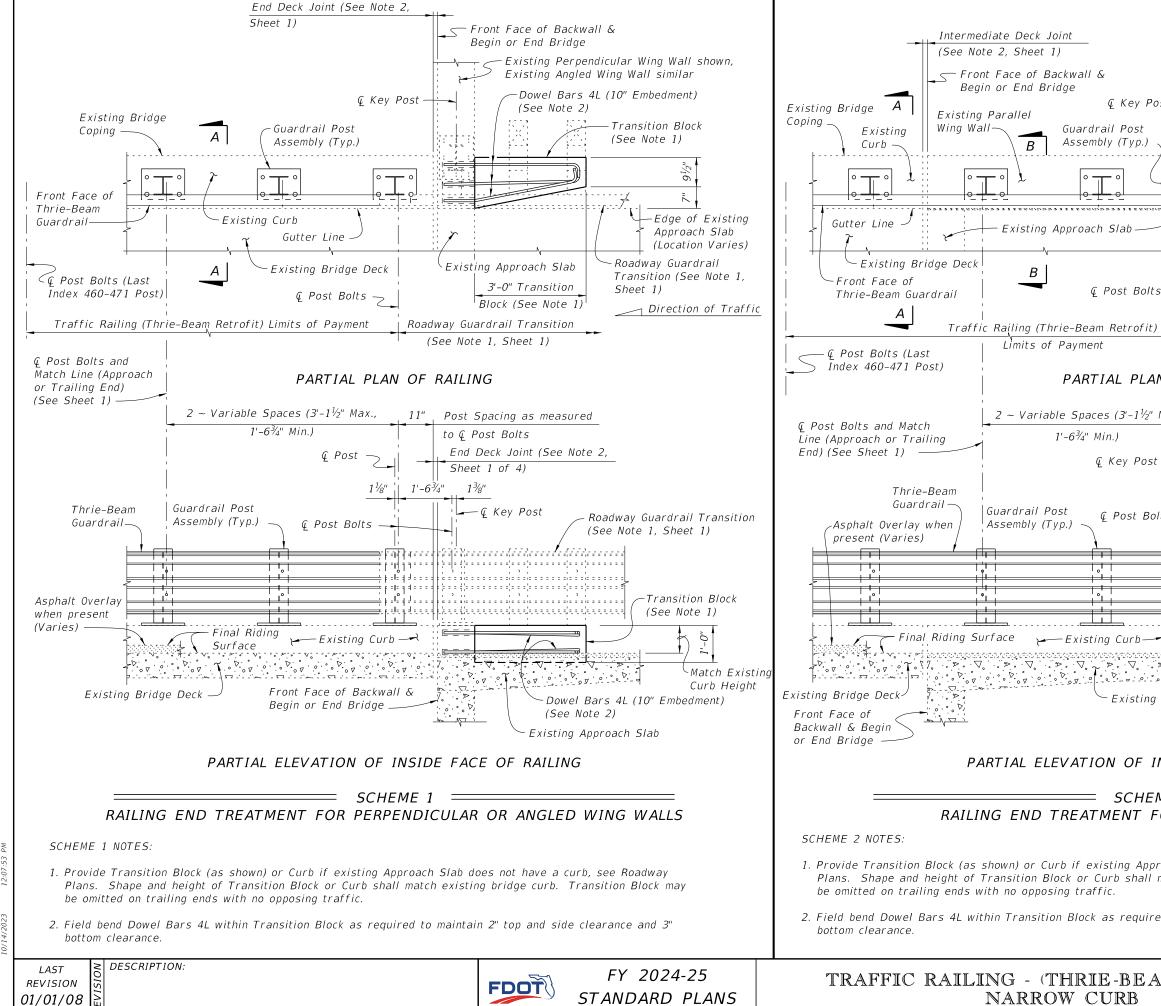
FDOT

FY 2024-25 STANDARD PLANS

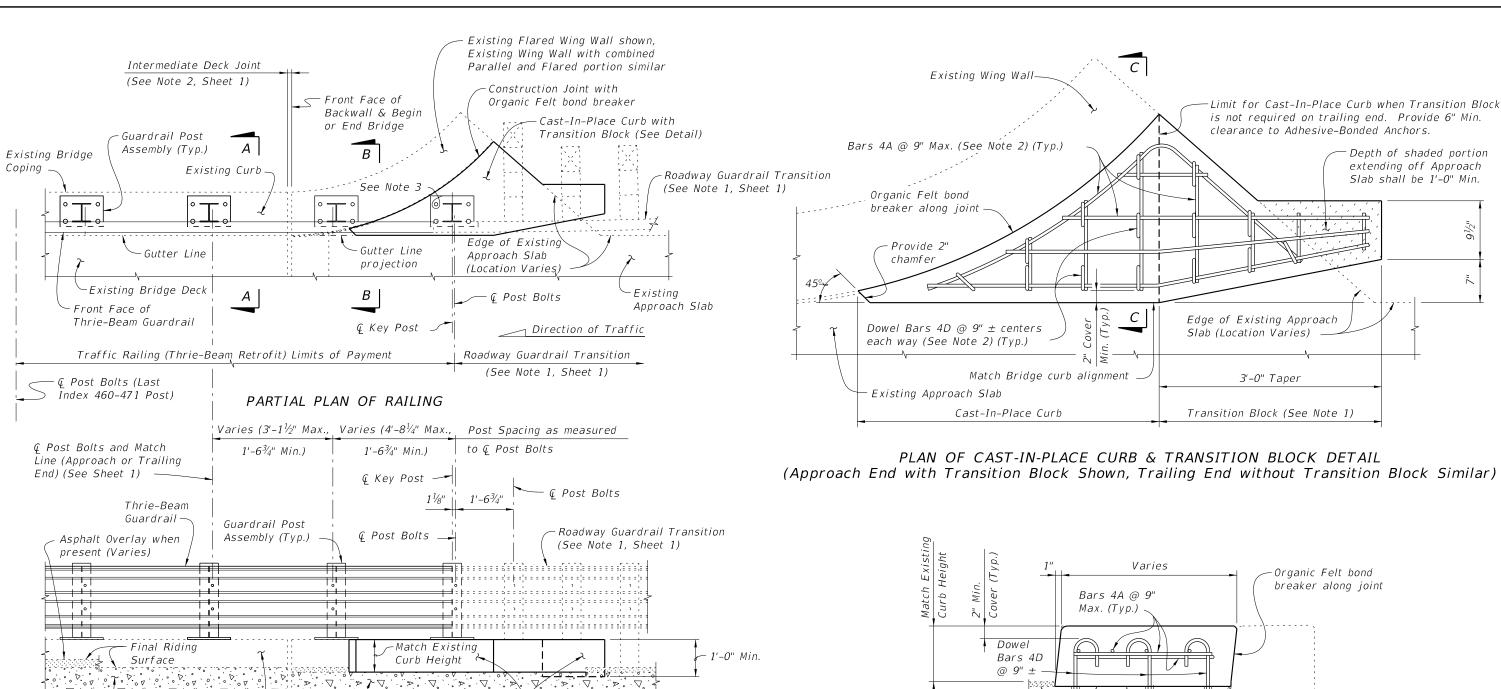
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

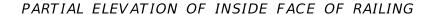
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Dowel Bars 4L (10" Embedment) (See Note 2) Transition Block (See Note 1) $9^{l/2}$ " Edge of Existing Approach Slab (Location Varies) Roadway Guardrail Transition (See Note 1, Sheet 1) € Post Bolts > 3'-0" Transition ____ Direction of Traffic Block (See Note 1) Roadway Guardrail Transition (See Note 1, Sheet 1) PARTIAL PLAN OF RAILING $2 \sim Variable Spaces (3'-1\frac{1}{2}'' Max., 11'' Post Spacing as measured$ to & Post Bolts ⊊ Key Post —__ End of Existing Wing Wall 11/8" 1'-6³/₄'' @ Post Bolts -Roadway Guardrail Transition (See Note 1, Sheet 1) و و و او او قالو و پ ± ji =||= j | | = = = ||= = = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= = ||= ||= = ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= ||= | -Transition Block (See Note 1) Existing Curb—— -Match Existing Curb Height Existing Approach Slab Dowel Bars 4L (10" Embedment) (See Note 2) PARTIAL ELEVATION OF INSIDE FACE OF RAILING *── SCHEME 2 =* RAILING END TREATMENT FOR PARALLEL WING WALLS 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3"





— *SCHEME 3* — RAILING END TREATMENT FOR FLARED WING WALLS

SCHEME 3 NOTES:

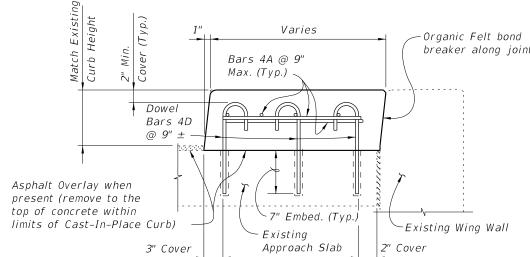
Existing

Bridge Deck -

1. Provide Cast-In-Place Curb as shown. Shape and height of Transition Block and Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic.

Existing Approach

- 2. Field cut and bend Bars 4A and rotate Dowel Bars 4B within Curb and Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. A single $\frac{7}{8}$ Ø x 8" Adhesive-Bonded Anchor may be omitted as shown when 2" clear cover cannot be provided.



SECTION C-C

DESCRIPTION: REVISION 11/01/16

Existing Curb

Begin or End Bridge —

Front Face of Backwall &



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Cast-In-Place Curb with

Transition Block (See Detail)

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

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NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

REVISION 01/01/08

FDOT

=== TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

FY 2024-25 STANDARD PLANS

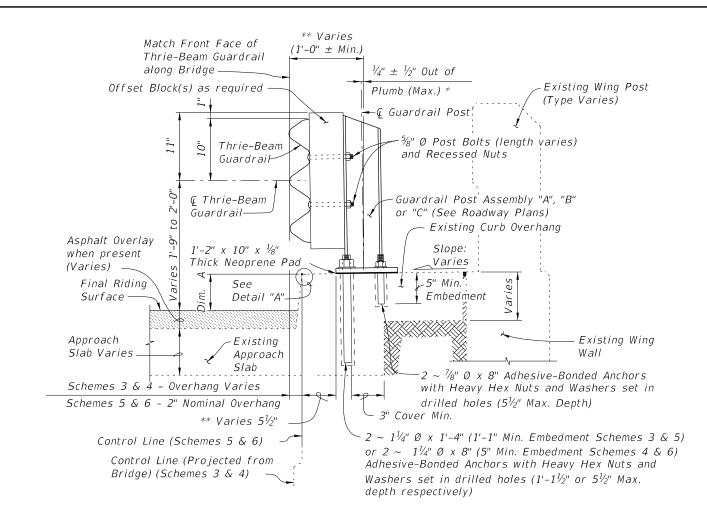
WIDE STRONG CURB TYPE 1

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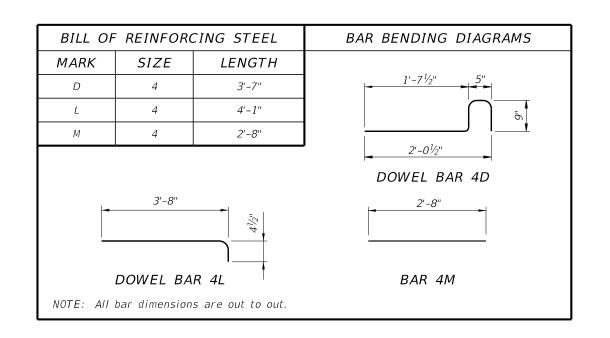
460-472 1 of 4

SHEET

SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

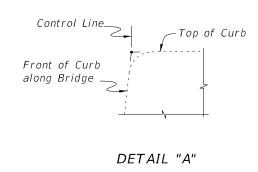


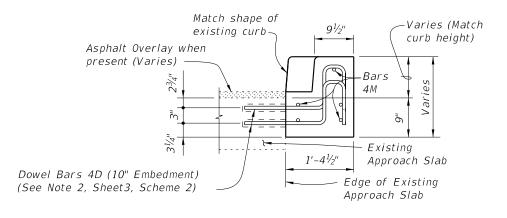
SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)



Shim with washers around Anchors as required to maintain tolerance.

Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 07/01/08

DESCRIPTION:

FDOT

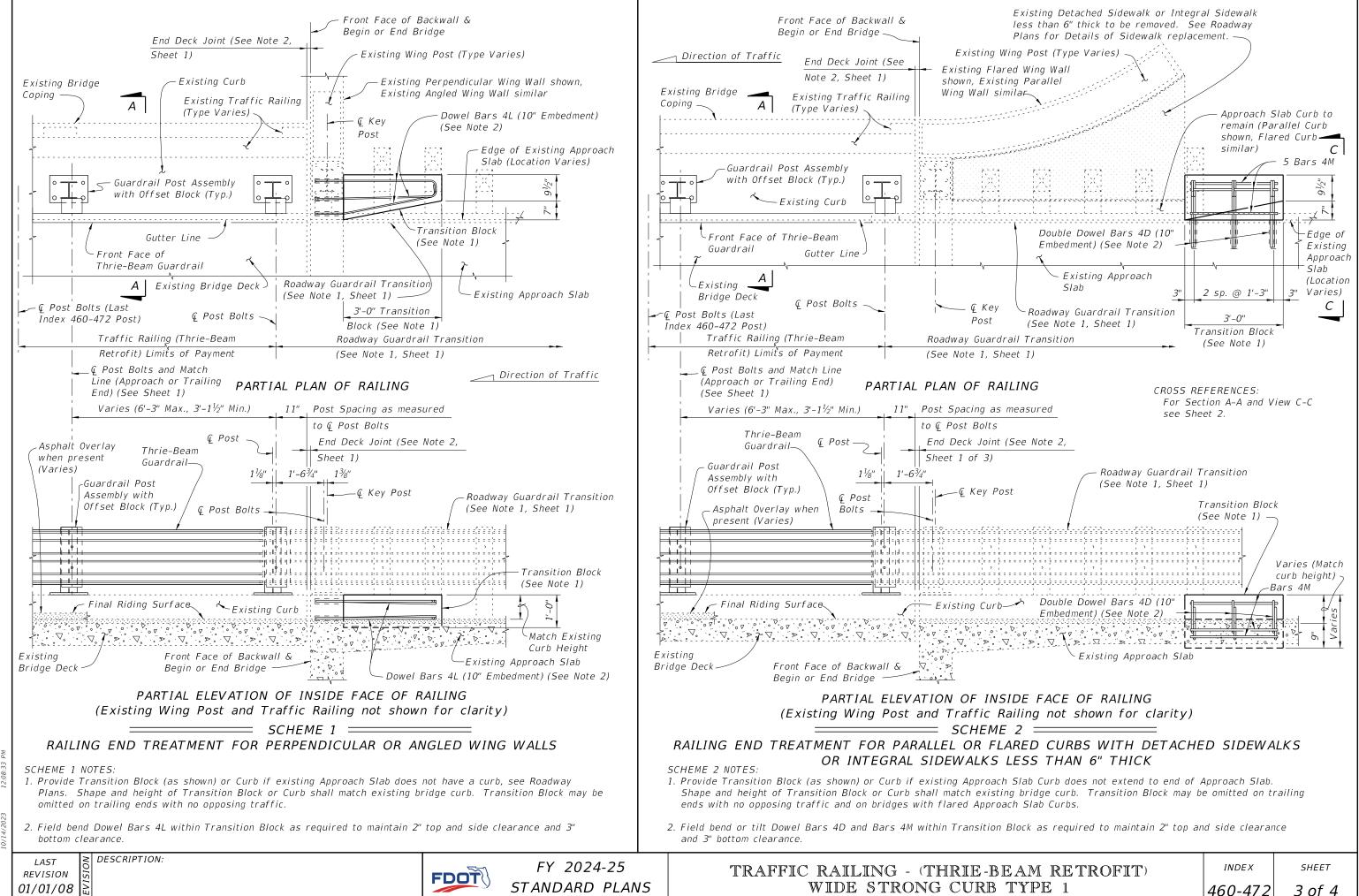
FY 2024-25 STANDARD PLANS

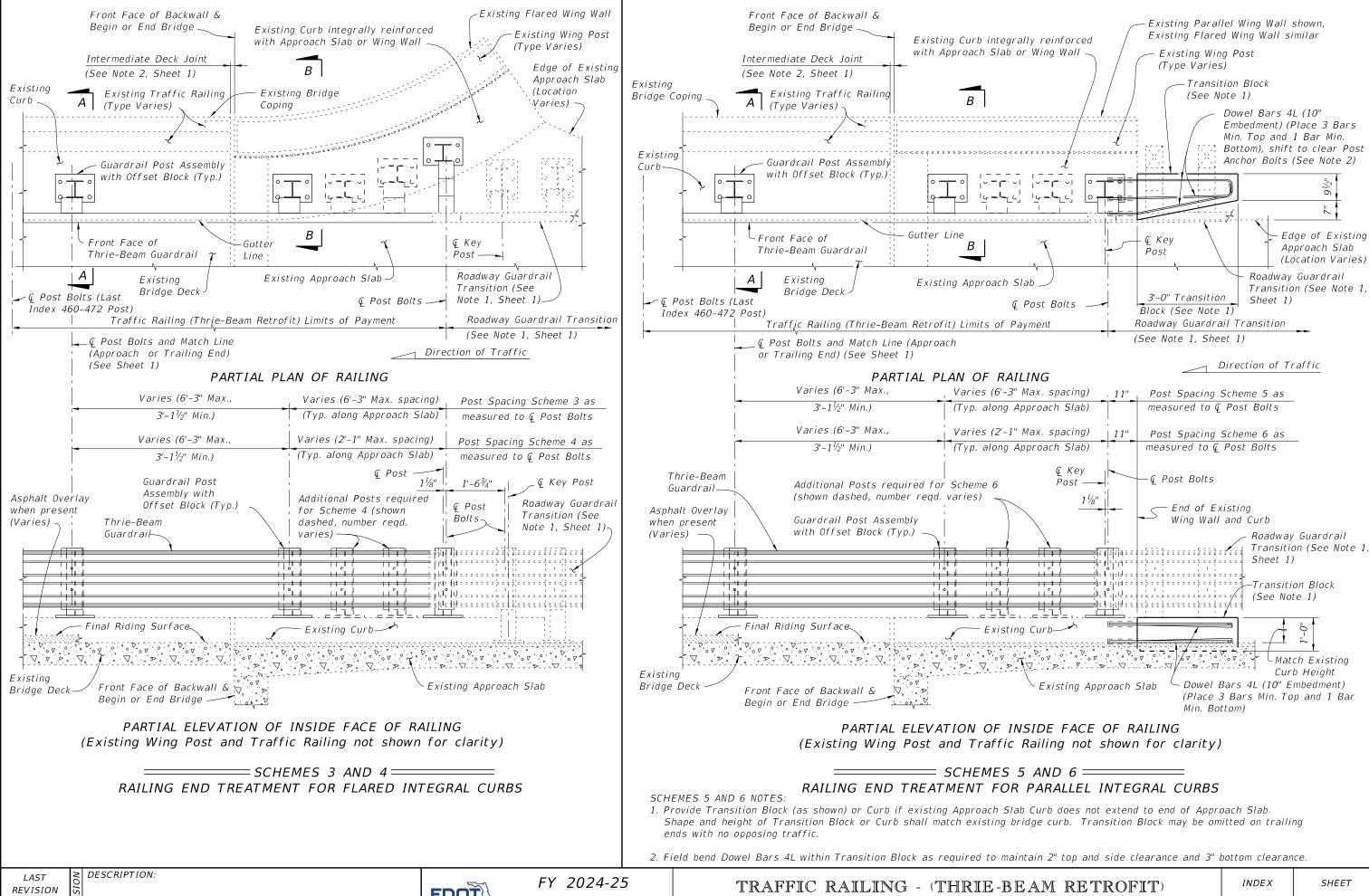
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 1

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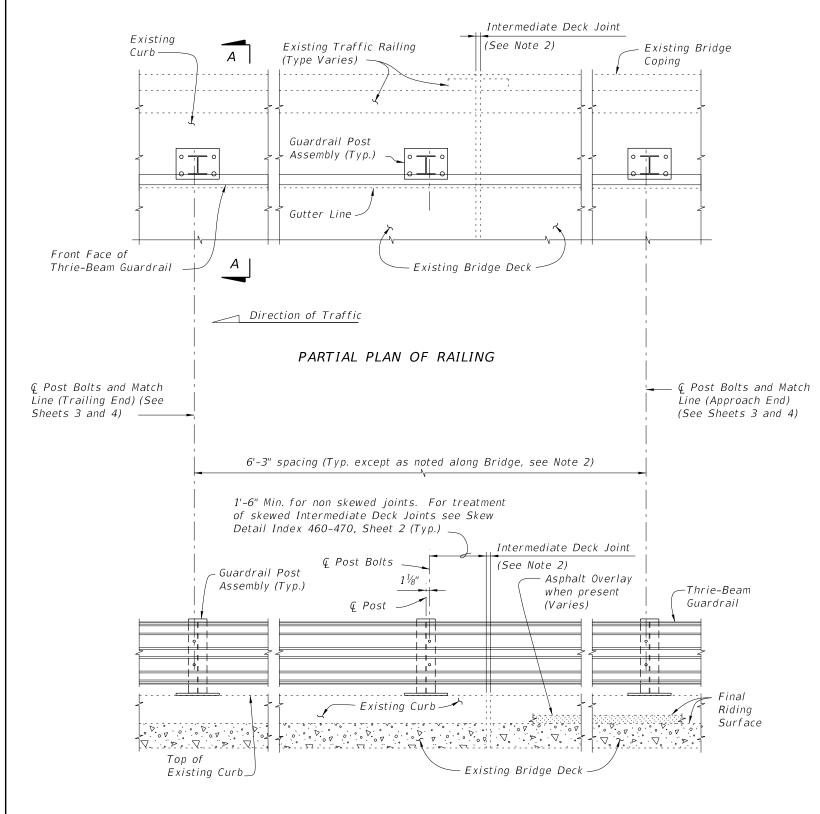
460-472





01/01/08

FDOT



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

10/14/2023

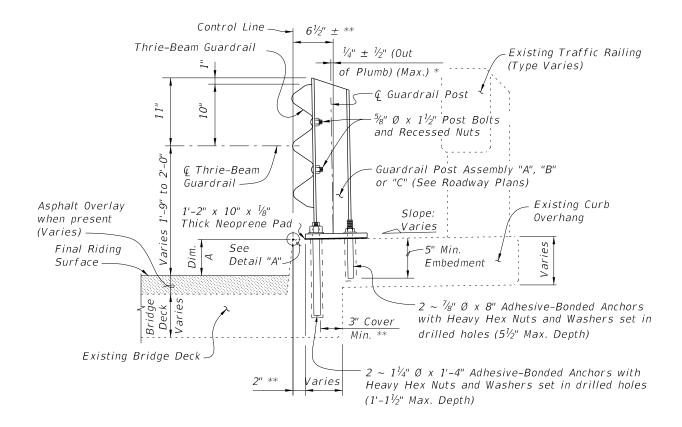
LAST REVISION 01/01/08

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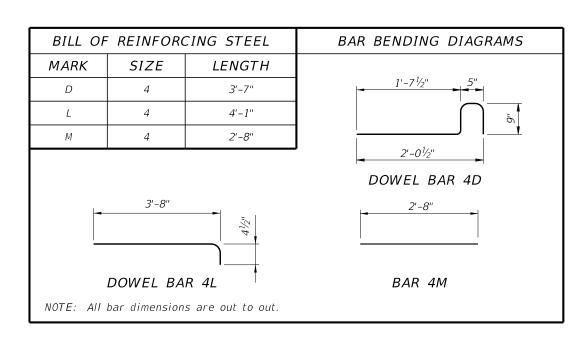
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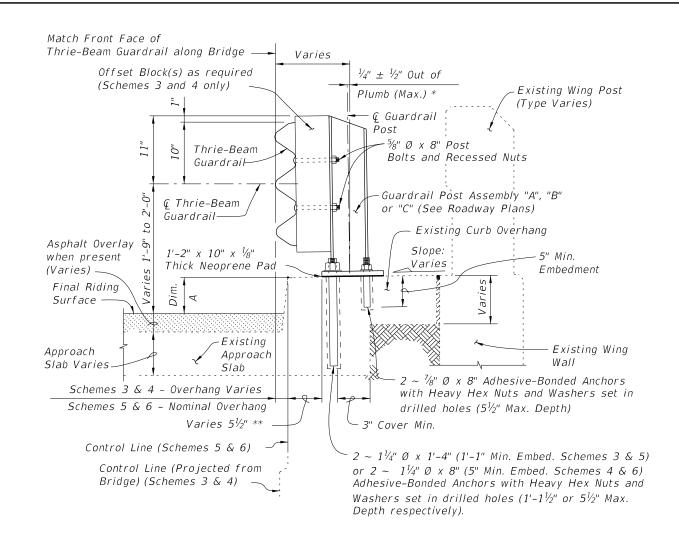
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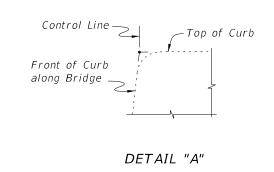
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

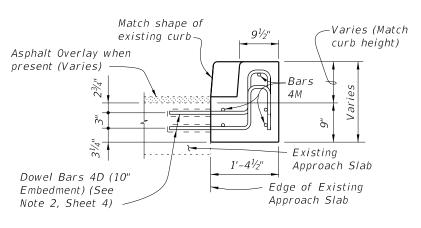




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- * Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- ** Offset may vary ± 1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index 460-470.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

DESCRIPTION: LAST REVISION 07/01/08

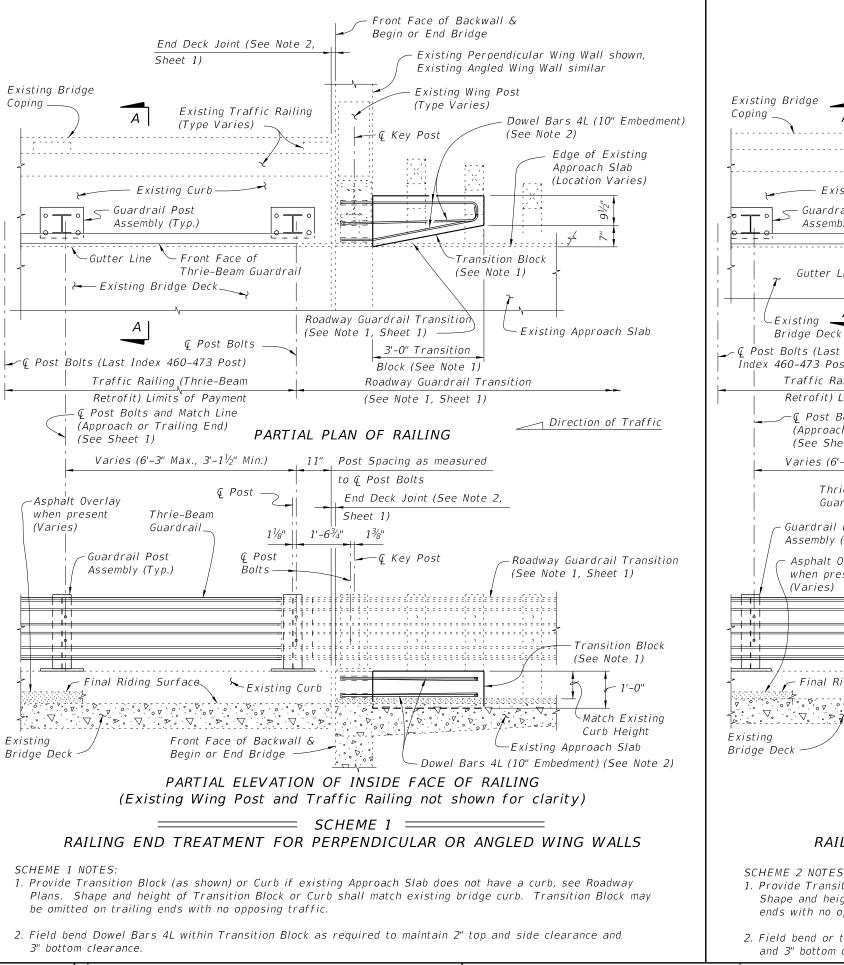
FDOT

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2

INDEX

SHEET

FY 2024-25 STANDARD PLANS



FY 2024-25 FDOT

Index 460-473 Post) Transition Block Traffic Railing (Thrie-Beam Roadway Guardrail Transition (See Note 1) Retrofit) Limits of Payment (See Note 1, Sheet 1) Edge of Existing Approach • © Post Bolts and Match Line Slab (Location Varies) (Approach or Trailing End) PARTIAL PLAN OF RAILING (See Sheet 1) ____ Direction of Traffic Varies (6'-3" Max., $3'-1\frac{1}{2}$ " Min.) 11" Post Spacing as measured to © Post Bolts Thrie-Beam End Deck Joint (See Note 2, € Post — Guardrail_ Sheet 1) Guardrail Post 11/8" 1'-6¾" 1¾" Assembly (Typ.) Asphalt Overlay Roadway Guardrail Transition when present Bolts. (See Note 1, Sheet 1) (Varies) Transition Block (See Note 1) Varies (Match Curb Height)-ร้ององให้เกรายเกราร์การเห็นของอาการใกรเห็นการเกราะในของห้ายการเห็น เกรียบเกรียบการให้เกราะในการให้เกรา**ะ**ในการเ -Bars 4M Existing Curb Double Dowel Bars 4D (10" - Final Riding Surface Embedment) (See Note 2) آع و الحوالا لك و الحوالية الك و الحوالية Existing Existing Approach Slab Bridge Deck Front Face of Backwall & Begin or End Bridge — PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post and Traffic Railing not shown for clarity) === SCHEME 2 === RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALK LESS THAN 6" THICK SCHEME 2 NOTES: 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs. 2. Field bend or tilt Dowel Bars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance. *INDEX* SHEET TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2 460-473 3 of 4

Existing Detached Sidewalk or Integral Sidewalk

less than 6" thick to be removed. See Roadway

Approach Slab Curb to

shown, Flared Curb similar

C

C

5 Bars 4M

remain (Parallel Curb

2 sp. @ 1'-3"

3'-0"

Plans for Details of Sidewalk replacement.

Double Dowel Bars 4D (10"

Existing Approach

Embedment) (See Note 2)

Slab

Roadway Guardrail Transition

(See Note 1, Sheet 1)

Existing Flared Wing

Wall shown, Existing Parallel Wing Wall similar

Existing Wing Post

(Type Varies)

Front Face of Backwall &

End Deck Joint (See

-Front Face of Thrie-Beam

Post —

Guardrail

© Post Bolts

Note 2, Sheet 1)

Existing Traffic Railing

(Type Varies)

Begin or End Bridge

Existing Bridge

─ Guardrail Post

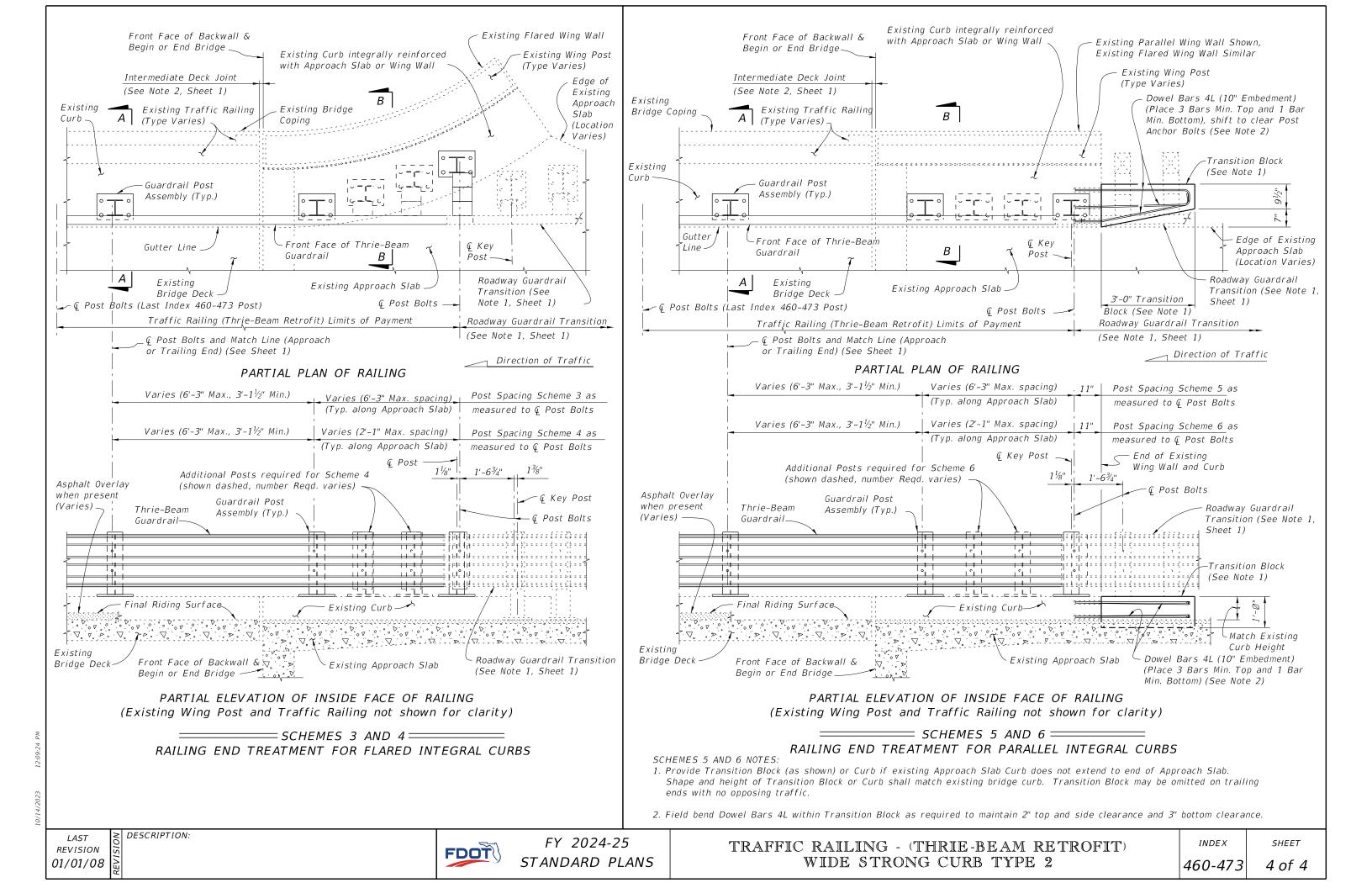
Gutter Line

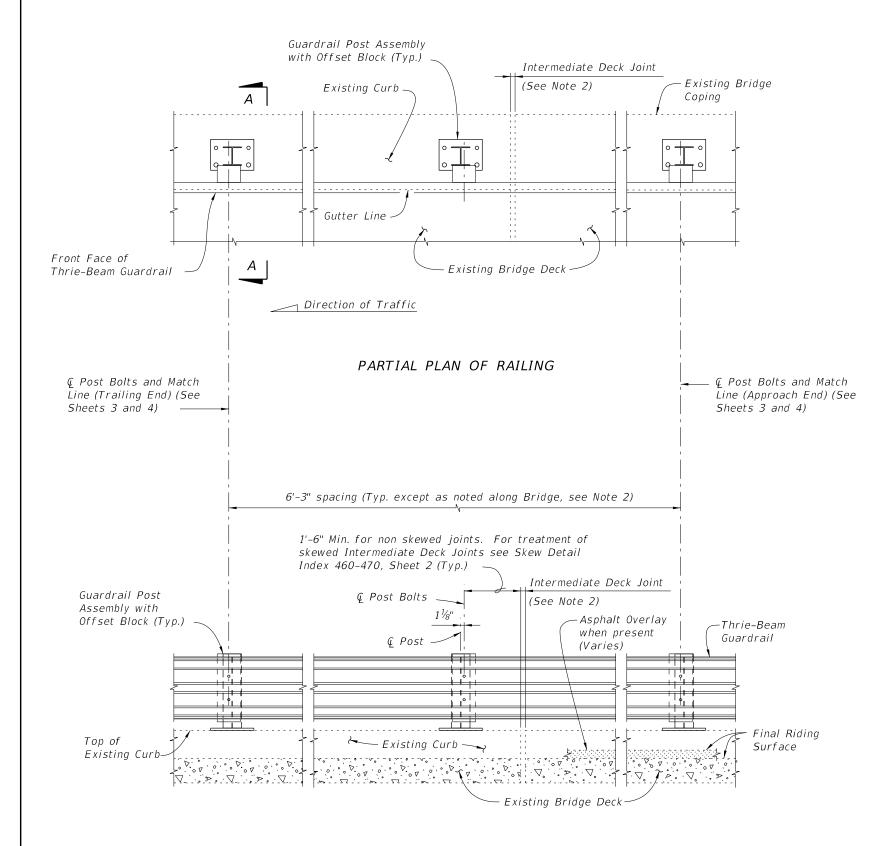
_Existing

Bridge Deck

Assembly (Typ.)

01/01/08





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460–470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Match Line see Sheets 3 & 4.
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details see
Index 460-470.

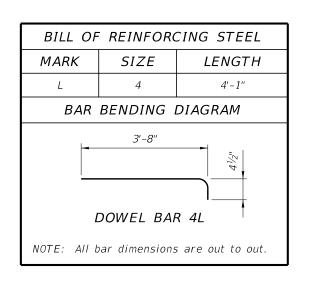
TIPICAL TREATMENT OF RAILING ALONG BRIDGE

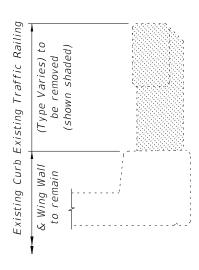
DESCRIPTION:

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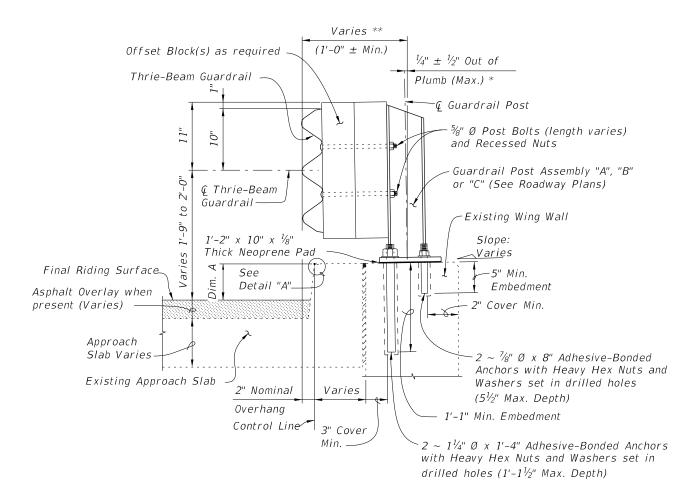
01/01/08







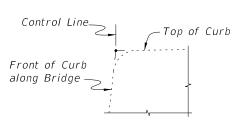
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION B-B (SCHEME 2) TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

 ** Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



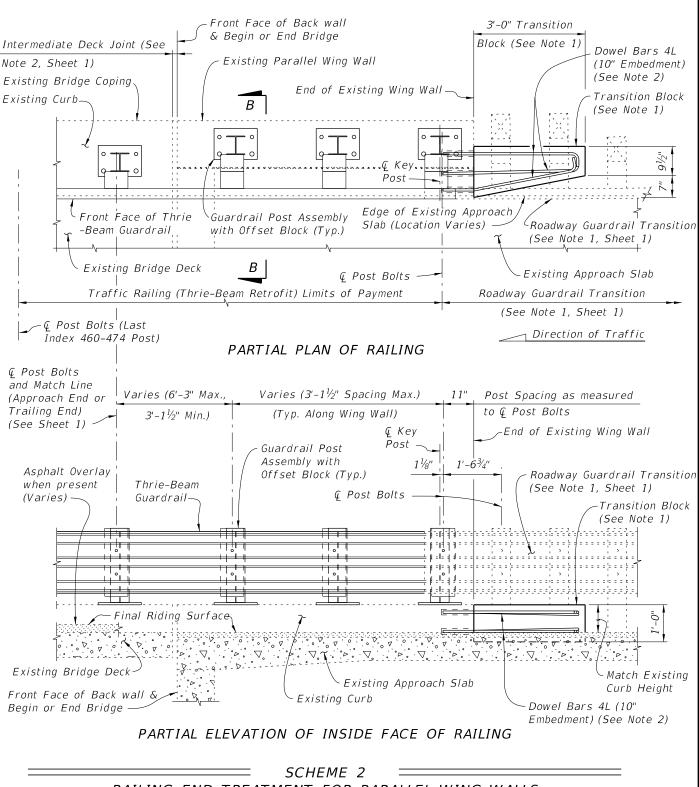
DETAIL "A"

CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3. For location of Section B-B see Sheet 3 For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

DESCRIPTION:

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RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES.

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

DESCRIPTION:

FY 2024-25 FDOT

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) INTERMEDIATE CURB

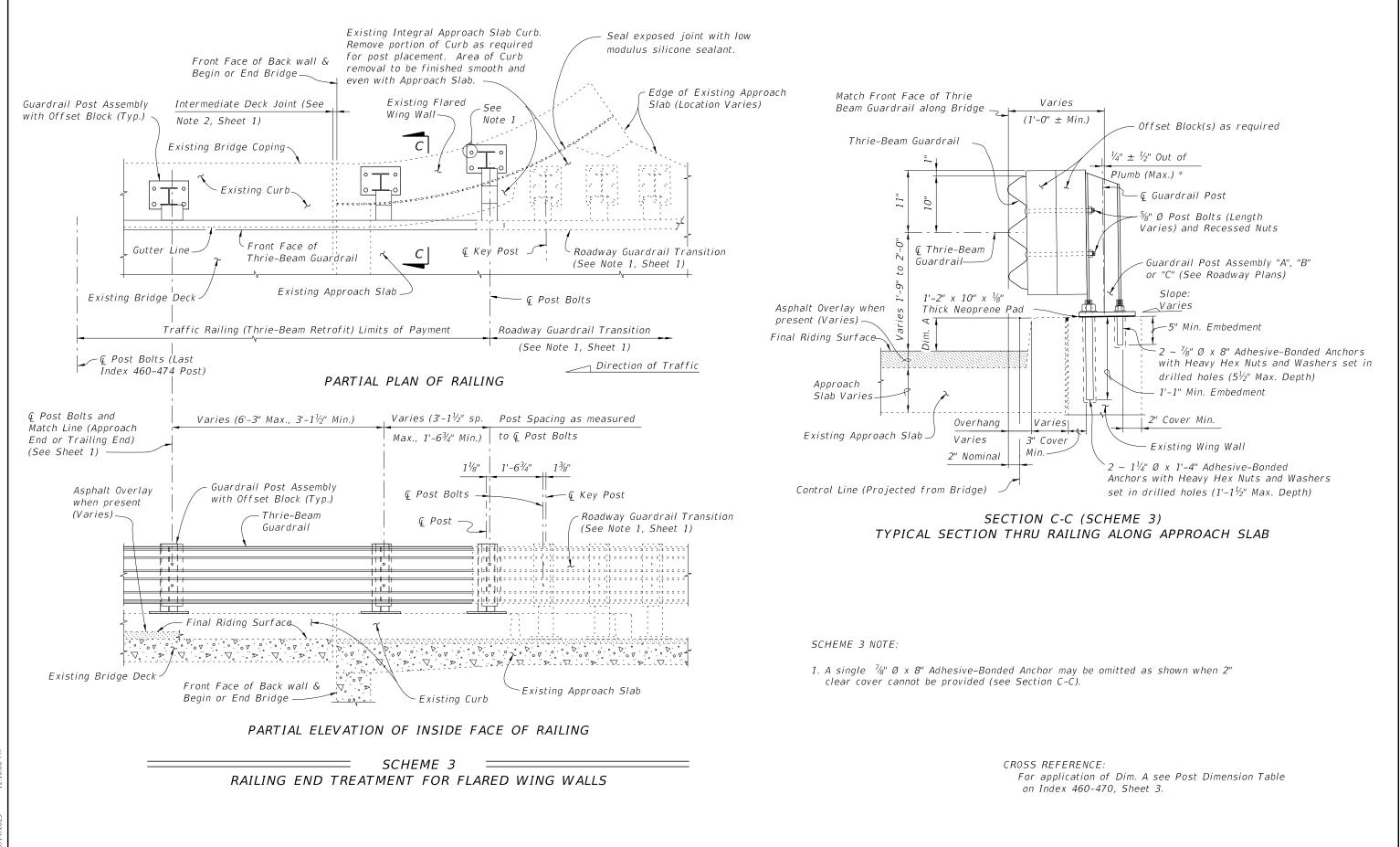
INDEX

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DESCRIPTION:

FDOT

PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

FDOT

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

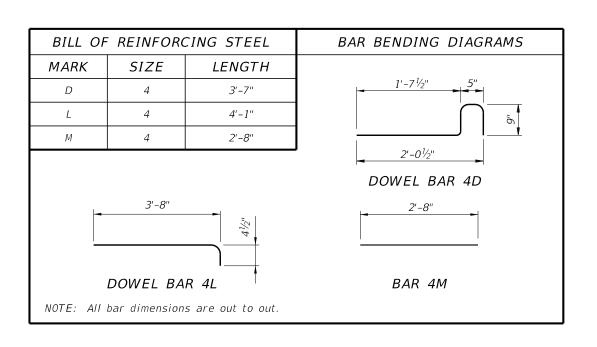
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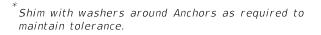
SHEET

460-475 1 of 4

DESCRIPTION:

SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

when present

Surface -

Approach

Slab Varies

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

Control Line (Schemes 5 & 6)

Bridge) (Schemes 3 & 4)

Control Line (Projected from

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required

Thrie-Beam

Guardrail-

 ← Thrie-Beam

1'-2" x 10" x 1/8"

See

Thick Neoprene Pad

Detail "A"

-Existing

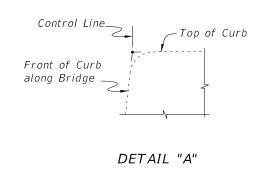
Approach

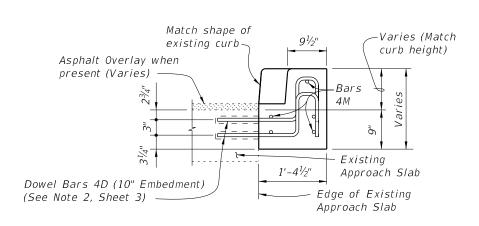
Slab

Varies 5½" **

Guardrail-

Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

Varies **

 $(1'-0" \pm Min.)$

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

Plumb (Max.) *

🗕 🕻 Guardrail Post:

½" Ø Post Bolts (length varies)

-Guardrail Post Assembly "A", "B"

Existing Curb Overhang

 $\sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5)

or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}")$ or $5\frac{1}{2}"$ Max.

drilled holes (5½" Max. Depth)

or "C" (See Roadway Plans)

and Recessed Nuts

Slope:

Varies

Embedment

.5" Min.

- 3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

Existing Wing Post

Existing Wing

(Type Varies)

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

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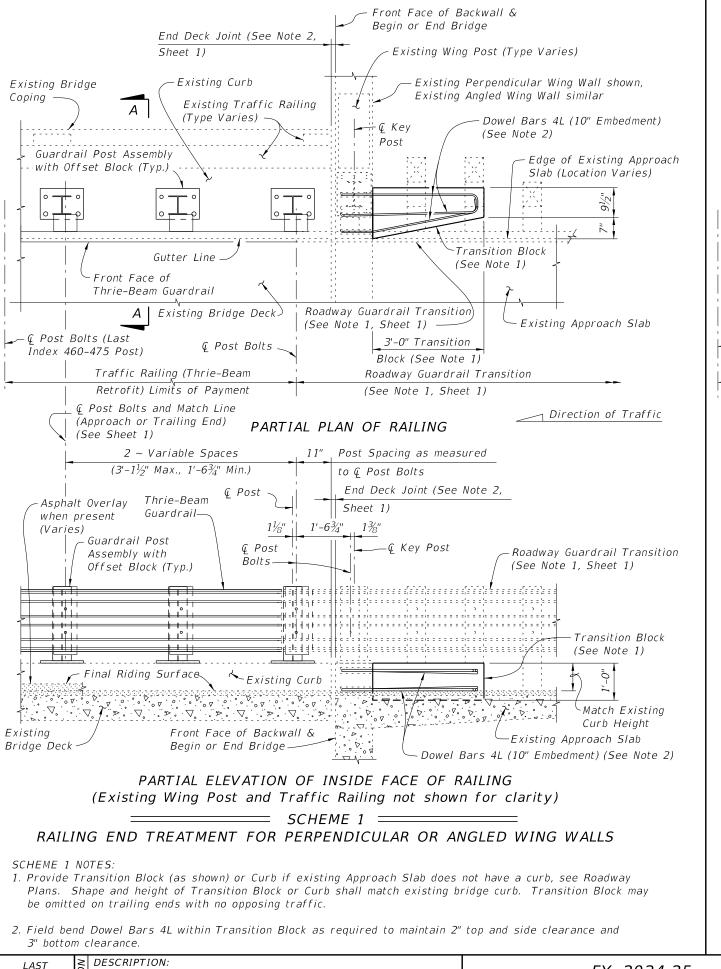
FY 2024-25 STANDARD PLANS

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

INDEX 460-475

SHEET 2 of 4

DESCRIPTION:

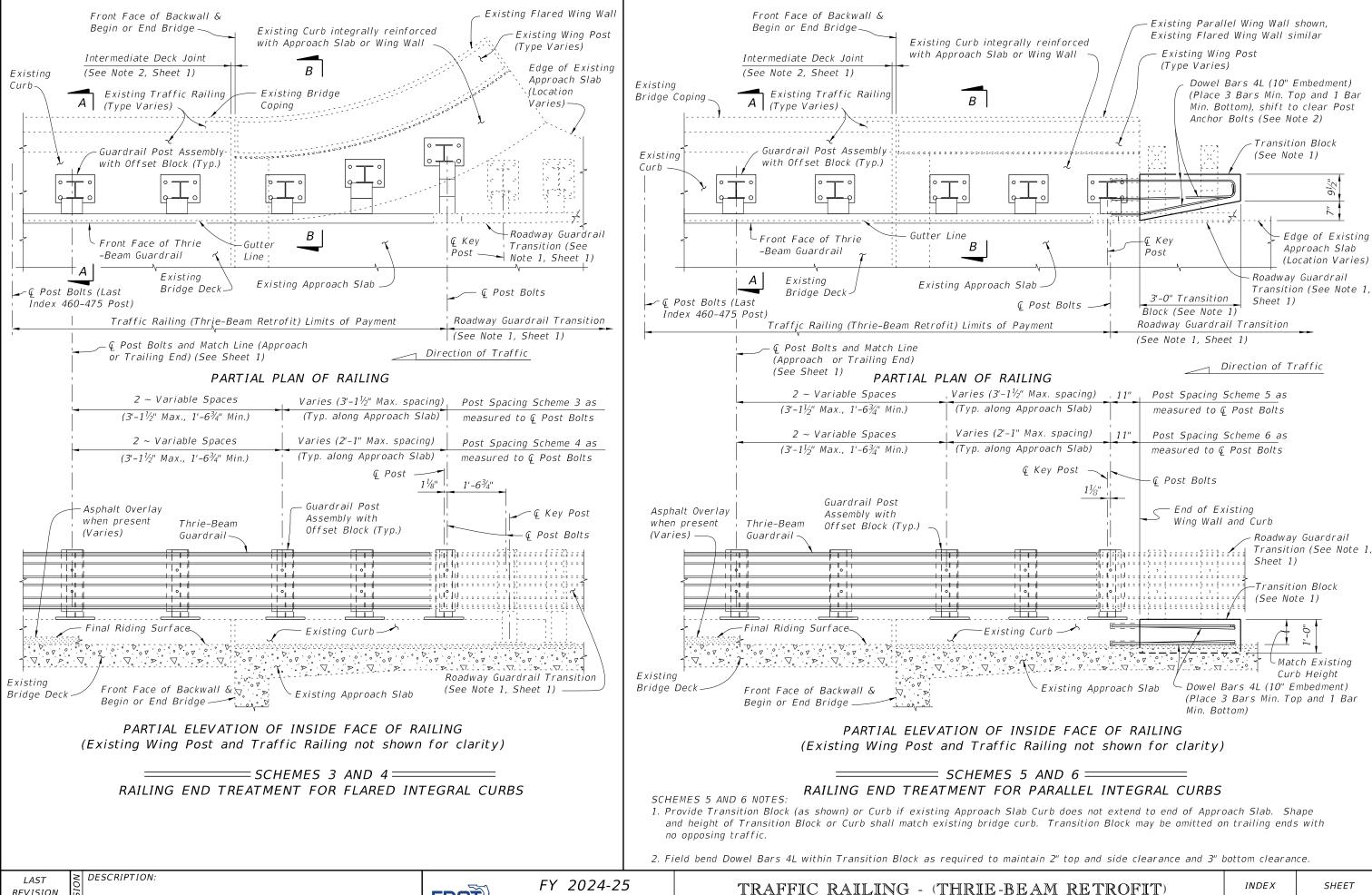


Existing Detached Sidewalk or Integral Sidewalk Front Face of Backwall & less than 6" thick to be removed. See Roadway Begin or End Bridge Plans for Details of Sidewalk replacement. Existing Wing Post (Type Varies) End Deck Joint (See Existing Flared Wing Wall Note 2, Sheet 1) shown, Existing Parallel Existing Bridge Wing Wall similar Existing Traffic Railing Approach Slab Curb to (Type Varies) remain (Parallel Curb shown, Flared Curb similar) Guardrail Post Assembly Cwith Offset Block (Typ.) 5 Bars 4M – Existing Curb 91/2" Double Dowel Bars 4D (10" Edge of Front Face of Thrie Embedment) (See Note 2) Existing -Beam Guardrail Gutter Line Approach Slab Existing Approach (Location \sim Existing -Varies) 2 sp. @ 1'-3" − ¢ Key Bridge Deck @ Post Bolts -С Roadway Guardrail Transition Post Bolts (Last 3'-0" Index 460-475 Post) (See Note 1, Sheet 1) Transition Block Traffic Railing (Thrie-Beam Roadway Guardrail Transition (See Note 1) Retrofit) Limits of Payment (See Note 1, Sheet 1) © Post Bolts and Match Line ____ Direction of Traffic (Approach or Trailing End) PARTIAL PLAN OF RAILING (See Sheet 1) 11" Post Spacing as measured 2 ~ Variable Spaces $(3'-1\frac{1}{2}'')$ Max., $1'-6\frac{3}{4}''$ Min.) to © Post Bolts ℚ Post End Deck Joint (See Note 2, Thrie-Beam Sheet 1) Guardrail Post Guardrail-1'-6¾'' Assembly with Offset Block (Typ.) __@ Key Post ₽ Post Asphalt Overlay when Roadway Guardrail Transition present (Varies) (See Note 1, Sheet 1) Transition Block (See Note 1) Varies (Match ي يا ليا يا إلى " التي Curb Height)--Bars 4M Existing Curb Double Dowel Bars 4D (10" - Final Riding Surface Embedment) (See Note 2) اً عِنْ الْمُونَانِ الْمُونَانِ الْمُونَانِ الْمُونِانِ الْمُونِانِ الْمُونِانِ الْمُونِانِ الْمُونِ Existing `Existing Approach Slab Bridge Deck Front Face of Backwall & 🗸 🗸 😕 Begin or End Bridge—— PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post and Traffic Railing not shown for clarity) ______ SCHEME 2 ___ RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALKS LESS THAN 6" THICK SCHEME 2 NOTES: 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs. 2. Field bend or tilt Dowel Bars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance. *INDEX* SHEET TRAFFIC RAILING - (THRIE-BEAM RETROFIT)

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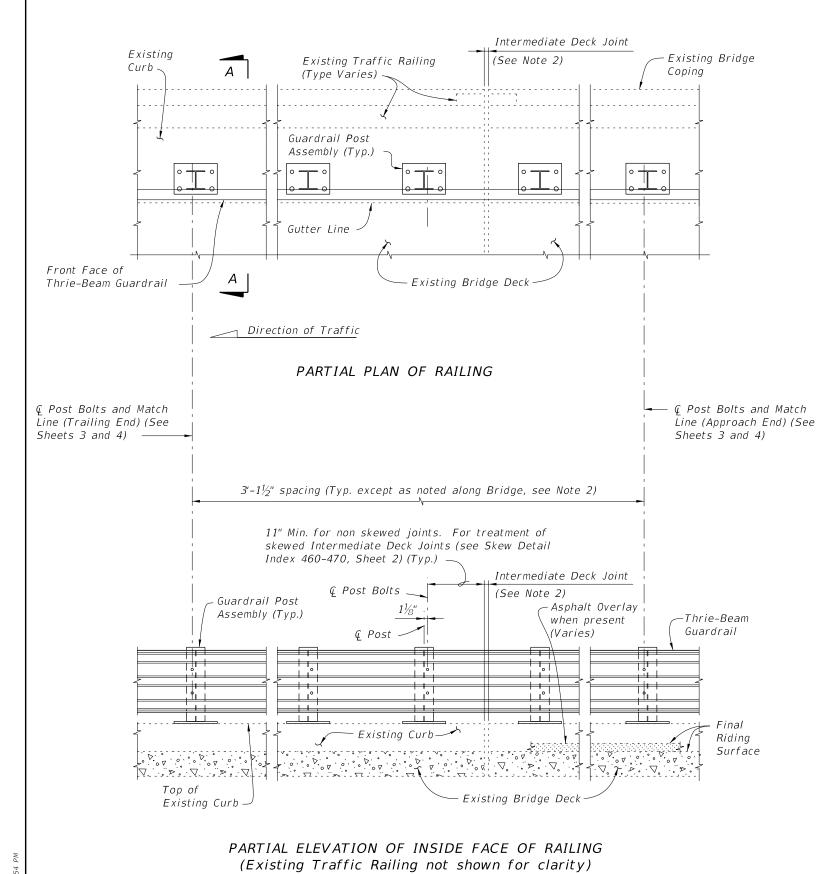
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REVISION 01/01/08

FDOT



TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

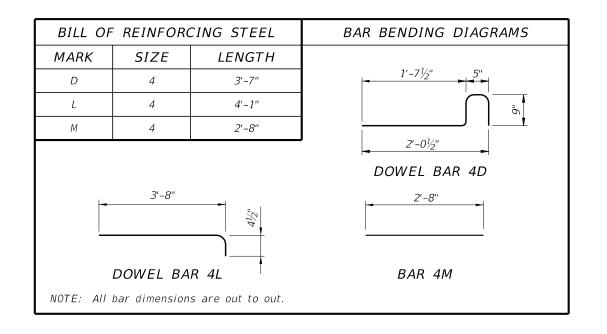
REVISION 01/01/08

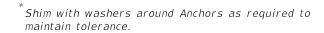
DESCRIPTION:

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FY 2024-25 STANDARD PLANS

SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

Surface

Approach

Slab Varies

when present

(Varies) -

Thrie-Beam Guardrail along Bridge 🥌

Offset Block(s) as required

Thrie-Beam

Guardrail-

 ← Thrie-Beam

 $1'-2'' \times 10'' \times \frac{1}{8}''$

Thick Neoprene Pad

-Existing

Varies 5½" **

Approach

Guardrail

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - Nominal Overhang

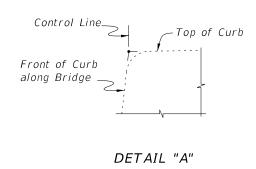
Control Line (Schemes 5 & 6)

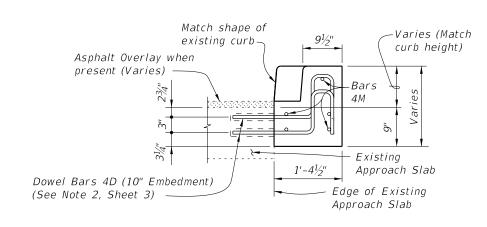
Control Line (Projected from

Bridge) (Schemes 3 & 4) -

(Schemes 3 and 4 only)

Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of Section C-C see Sheet 3.

 $\frac{1}{4}$ " ± $\frac{1}{2}$ " Out of

Plumb (Max.) *

3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively).

- Ç Guardrail Post

%" Ø x 8" Post

Slope:

Varies

Bolts and Recessed Nuts

or "C" (See Roadway Plans)

Guardrail Post Assembly "A", "B"

Existing Curb Overhang

____5" Min. ;

drilled holes (5½" Max. Depth)

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5) or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}" \text{ or } 5\frac{1}{2}" \text{ Max.}$

Embedment

 $\sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

Existing Wing Post

Existing Wing

(Type Varies)

For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

REVISION 07/01/08

FDOT

FY 2024-25 STANDARD PLANS

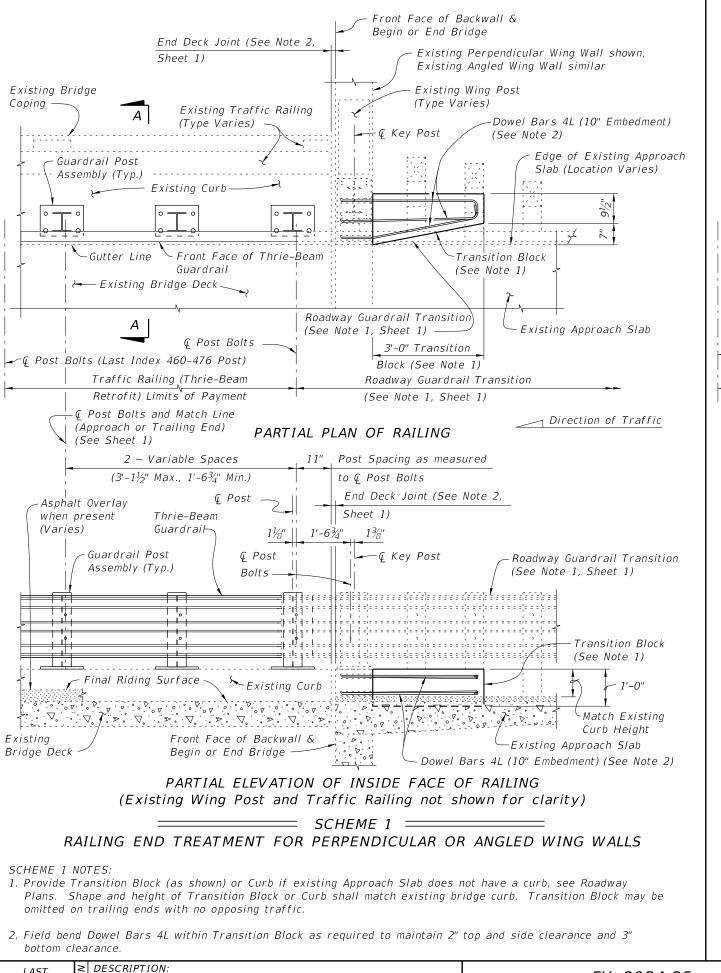
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 2

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SHEET

DESCRIPTION:

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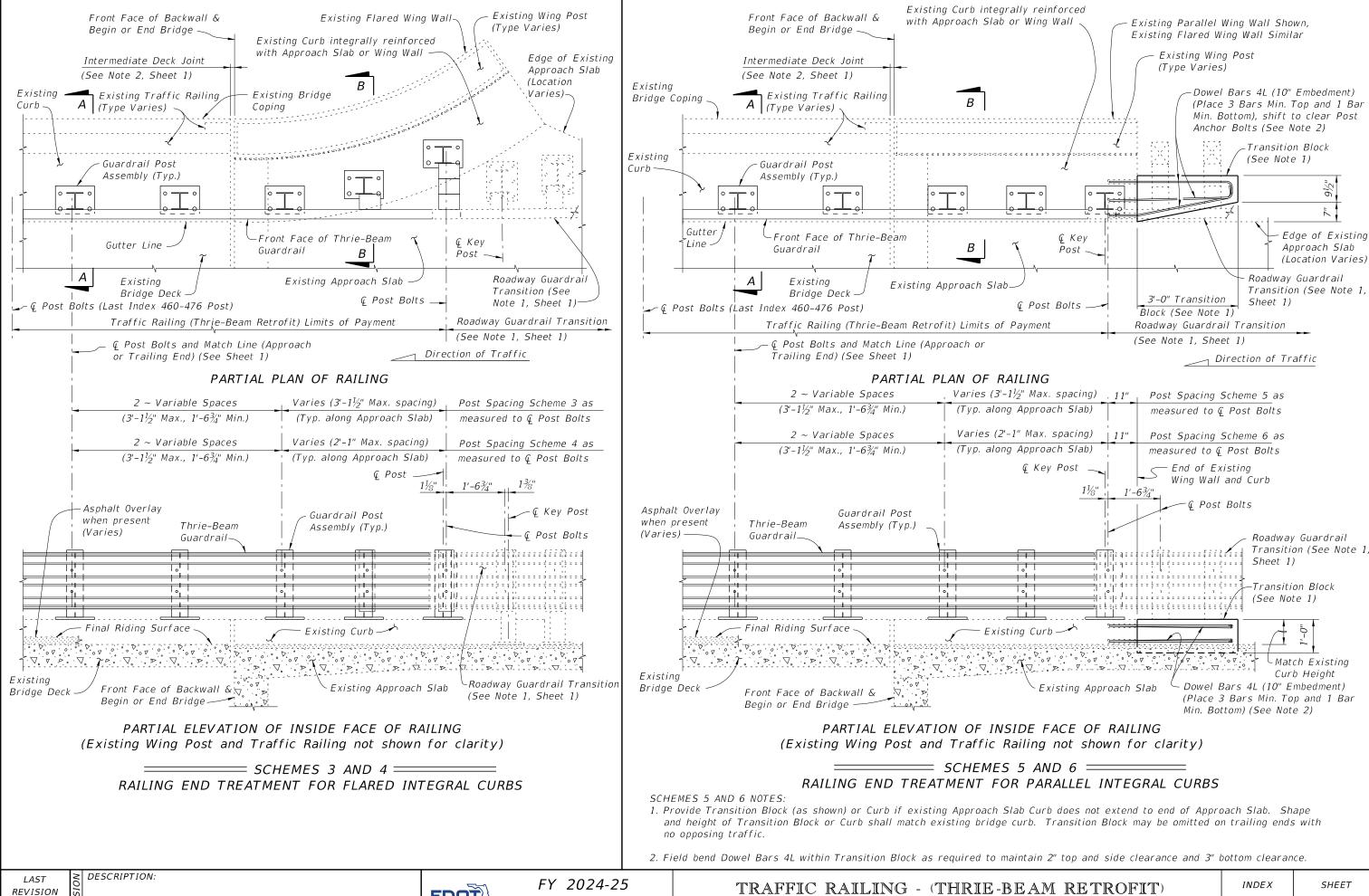
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Existing Detached Sidewalk or Integral Sidewalk Front Face of Backwall & less than 6" thick to be removed. See Roadway Begin or End Bridge Plans for Details of Sidewalk replacement. Existing Flared Wing End Deck Joint (See Wall shown, Existing Parallel Wing Wall similar Note 2, Sheet 1) Existing Bridge ____ Existing Wing Post Existing Traffic Railing Approach Slab Curb to (Type Varies) (Type Varies) remain (Parallel Curb shown, Flared Curb similar) CAssembly (Typ.) 5 Bars 4M — Existing Curb— Double Dowel Bars 4D (10" Embedment) (See Note 2) Gutter Line -C└ Front Face of Thrie-Beam Existing Approach -Existing Guardrail Slab _Ç Key 2 sp. @ 1'-3" Bridge Deck Post Bolts (Last Roadway Guardrail Transition Post © Post Bolts -3'-0" (See Note 1, Sheet 1) Index 460-476 Post) Transition Block Traffic Railing (Thrie-Beam Roadway Guardrail Transition (See Note 1) Retrofit) Limits of Payment (See Note 1, Sheet 1) Edge of Existing Approach © Post Bolts and Match Line Slab (Location Varies) (Approach or Trailing End) PARTIAL PLAN OF RAILING → Direction of Traffic (See Sheet 1) 2 ~ Variable Spaces 11" Post Spacing as measured $(3'-1\frac{1}{2}'')$ Max., $1'-6\frac{3}{4}''$ Min.) to & Post Bolts ⊈ Post End Deck Joint (See Note 2, Sheet 1) Thrie-Beam Guardrail Post Guardrail-1'-6¾" 1¾" Assembly (Typ.) Asphalt Overlay € Post Roadway Guardrail Transition when present Bolts. (See Note 1, Sheet 1) (Varies) -Transition Block (See Note 1) Varies (Match Curb Height) -Bars 4M Existing Curb Double Dowel Bars 4D (10" Final Riding Surface Embedment) (See Note 2) Existing Approach Slab Existing Front Face of Backwall & Bridge Deck Begin or End Bridge PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post and Traffic Railing not shown for clarity) ______ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALK LESS THAN 6" THICK SCHEME 2 NOTES: 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs. 2. Field bend or tilt Dowel Bars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance *INDEX* SHEET TRAFFIC RAILING - (THRIE-BEAM RETROFIT)

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460-476 4 of 4 CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be $\frac{3}{4}$ " by $2\frac{1}{7}$ " slotted holes.

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

COATINGS: All Nuts, Bolts, Anchors, and Washers shall be hot-dip galvanized in accordance with the Specifications.

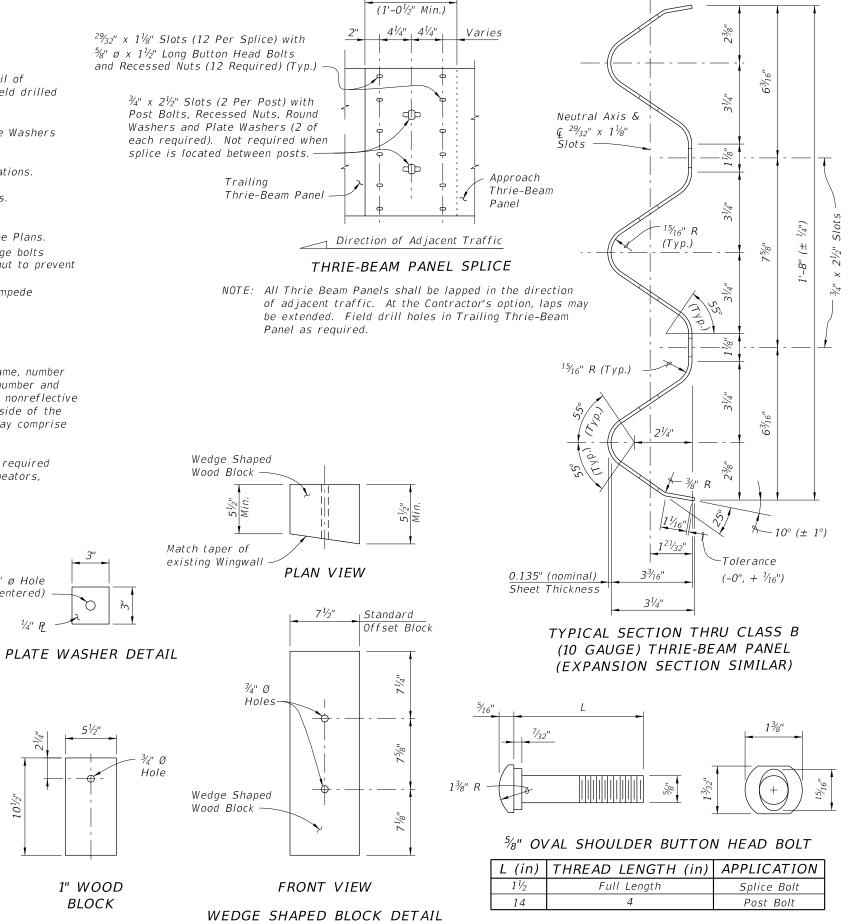
BRIDGES ON CURVED ALIGNMENTS: The details presented herein are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2^{1/2}$ " slots in thrie-beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten bolts in $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

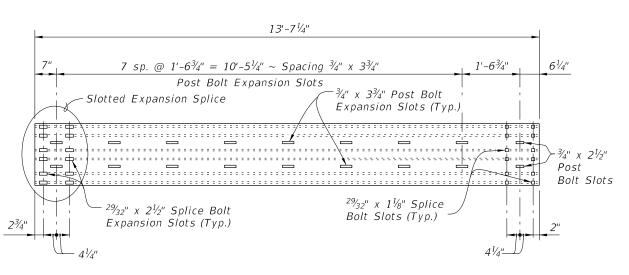
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered ($\pm^{1/4}$ ").

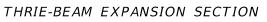
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie-Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

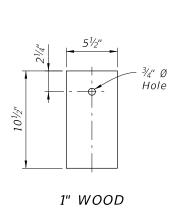
PAYMENT: Payment will be made under Thrie-Beam Panel Retrofit which shall include all materials and labor required to fabricate and install the retrofit railing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators, where required, will not be paid for directly but shall be considered incidental work.



Lap







¾" ø Hole

(centered)

REVISION 07/01/14

DESCRIPTION:

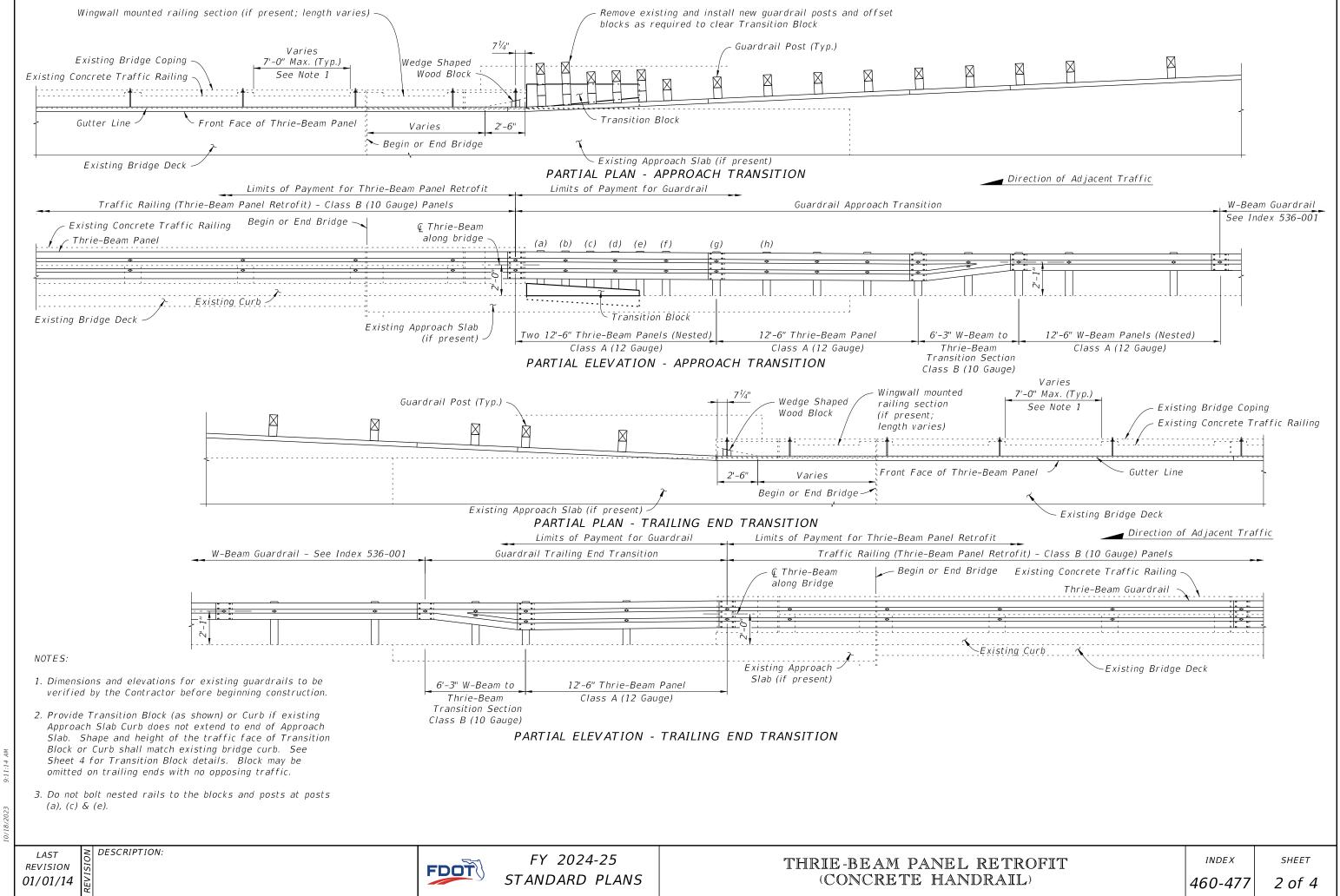


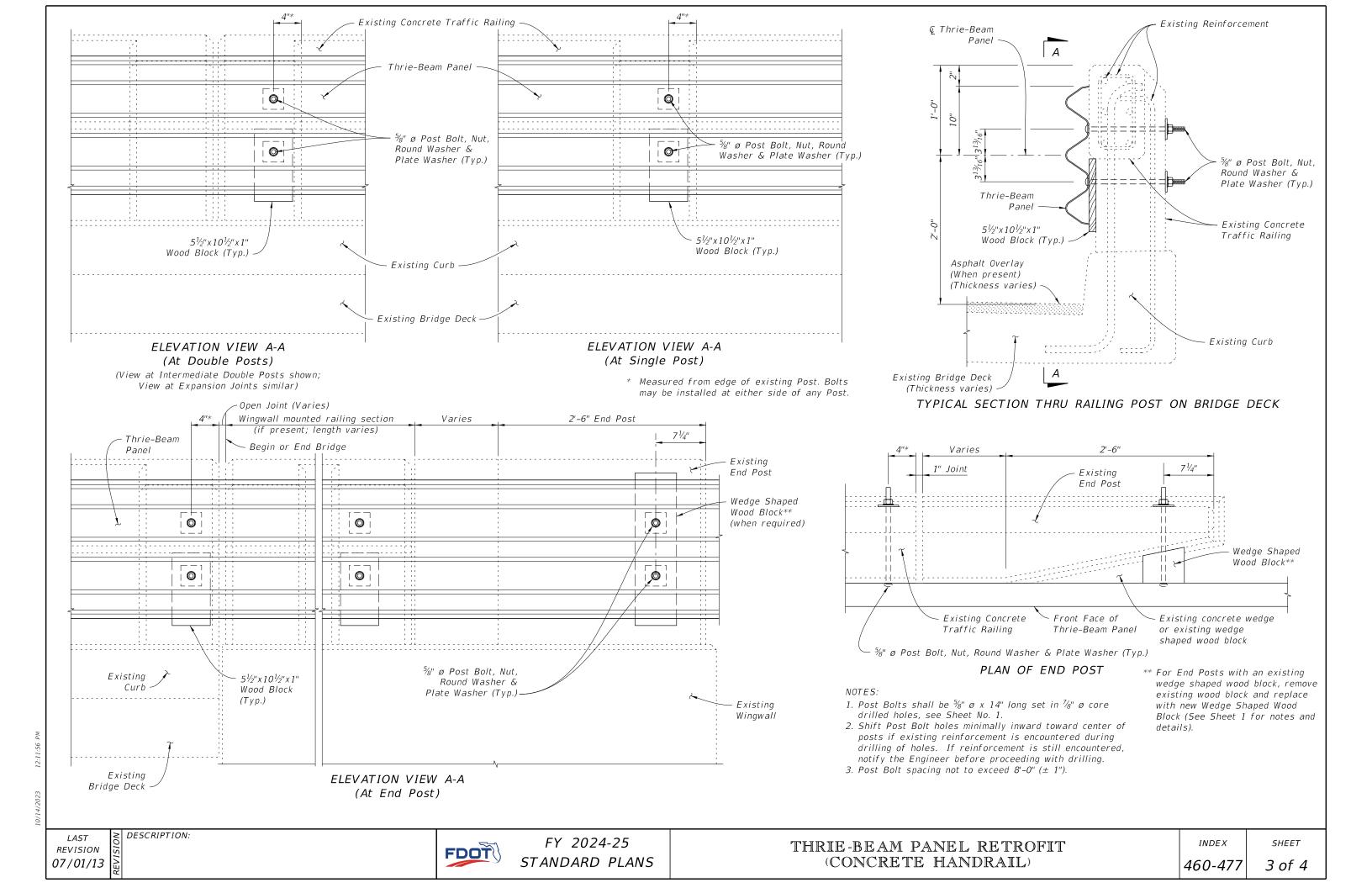
FY 2024-25 STANDARD PLANS

THRIE-BEAM PANEL RETROFIT (CONCRETE HANDRAIL)

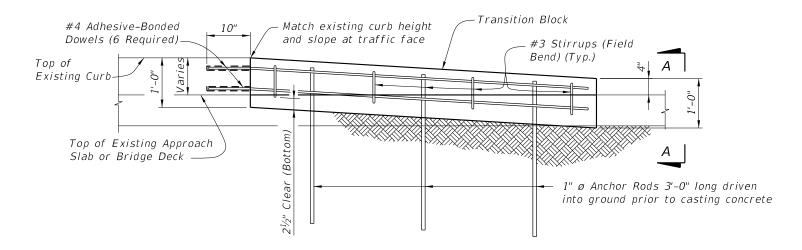
INDEX 460-477

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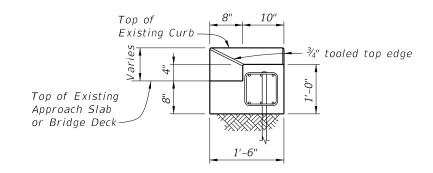


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

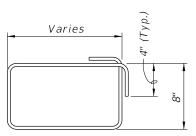


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK			
ITEM	UNIT	QUANTITY	
Concrete Class II (Bridge Deck)	CY	0.4	
Reinforcing Steel	LB	61	
Guardrail (Reset)	LF	12.5	



END VIEW A-A



#3 STIRRUP (FIELD BEND)

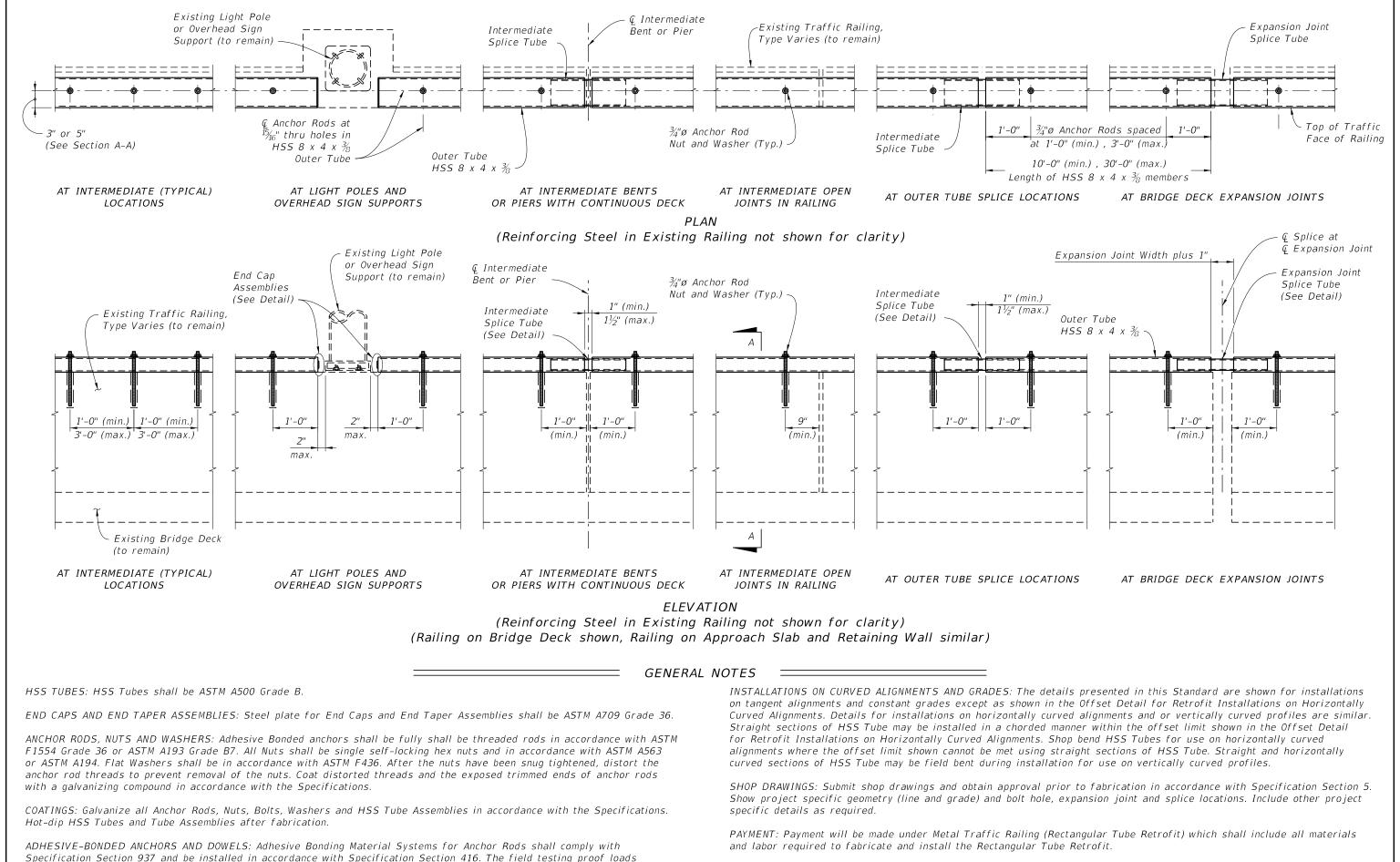
NOTES:

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 (Type HV) and be installed in accordance with Specification Section 416.

Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.



REVISION

11/01/17

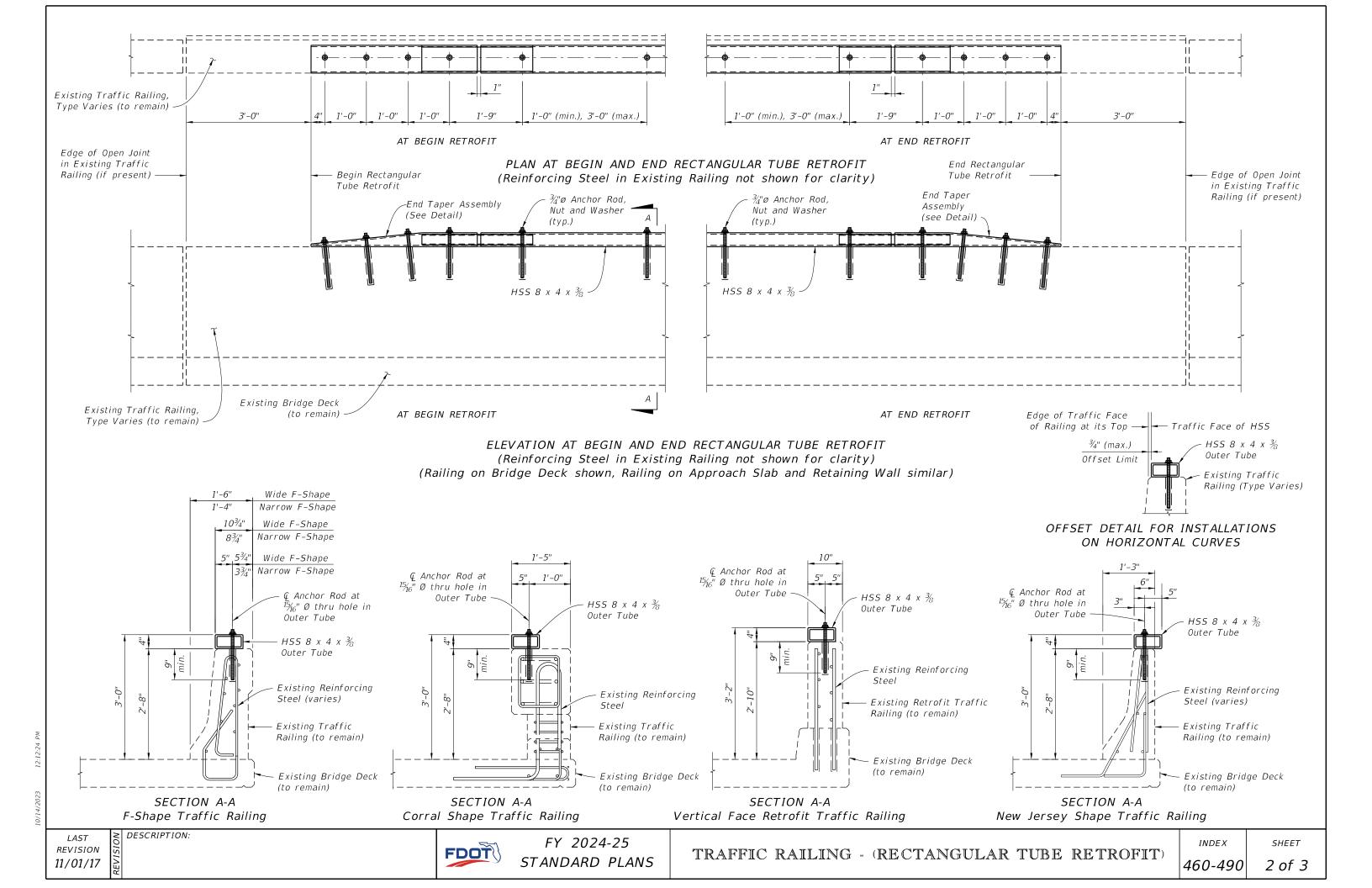
DESCRIPTION:

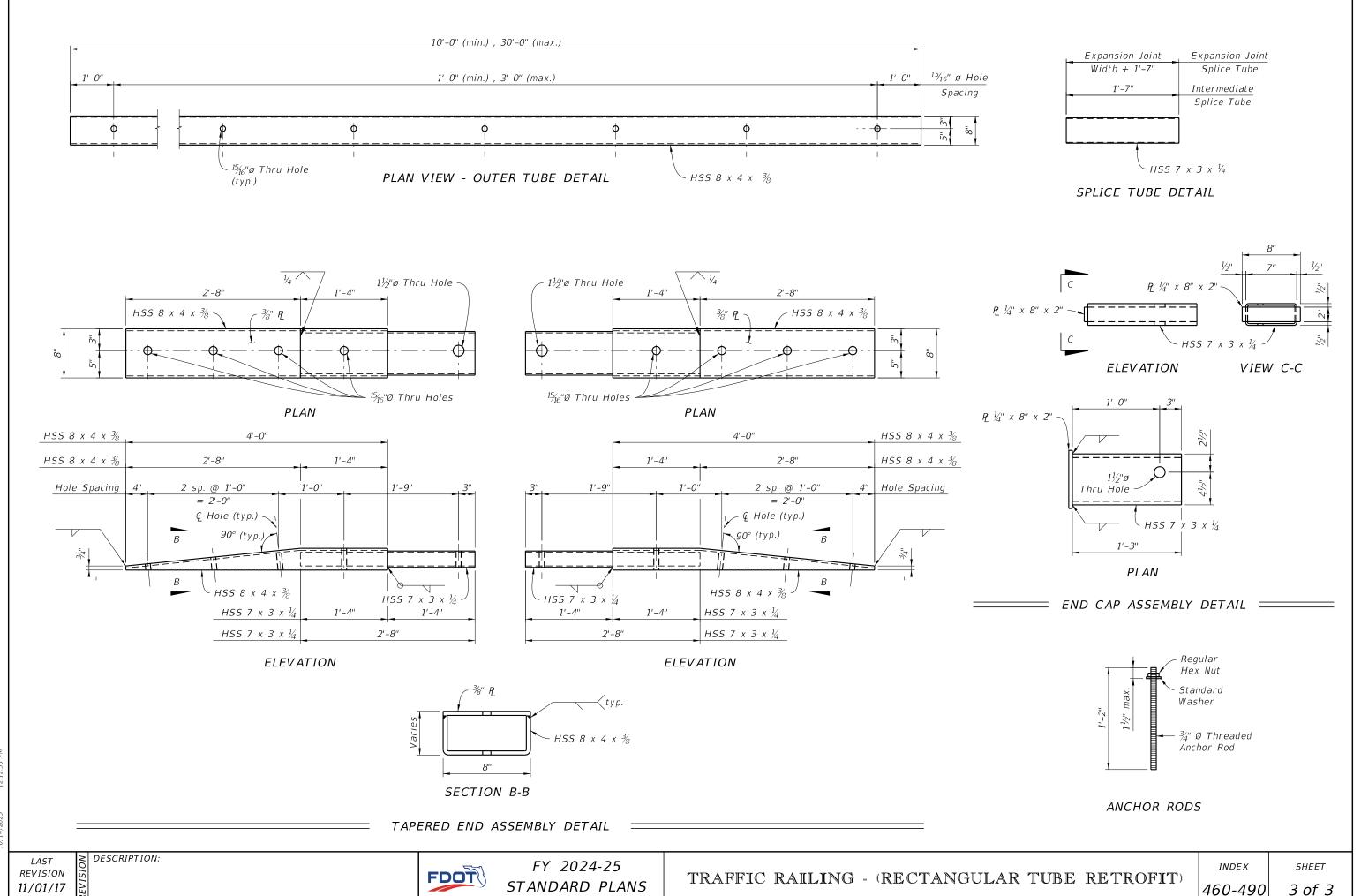
required by Specification Section 416 shall be 10,000 lbs.

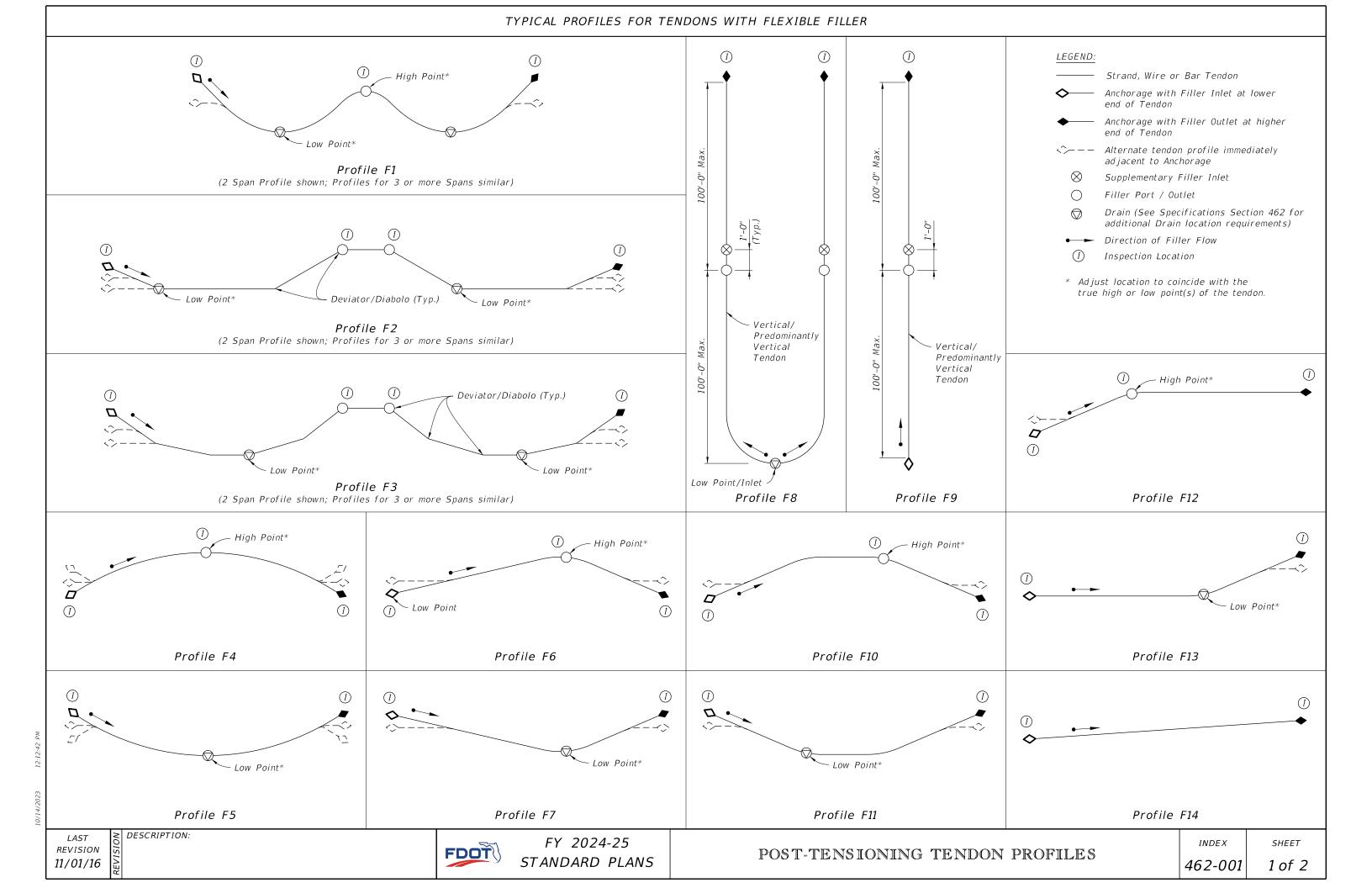
FY 2024-25 FDOT STANDARD PLANS

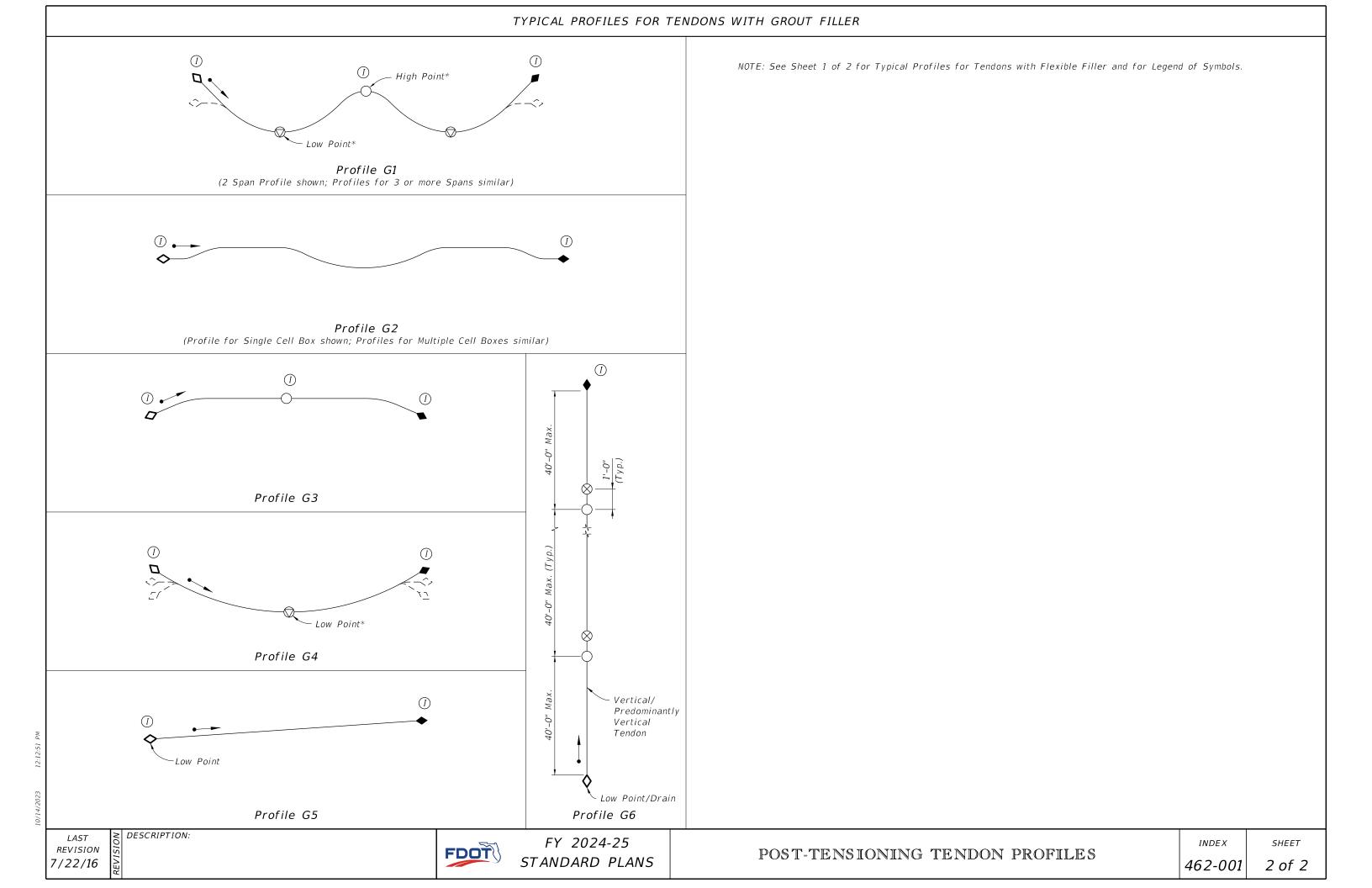
INDEX 460-490

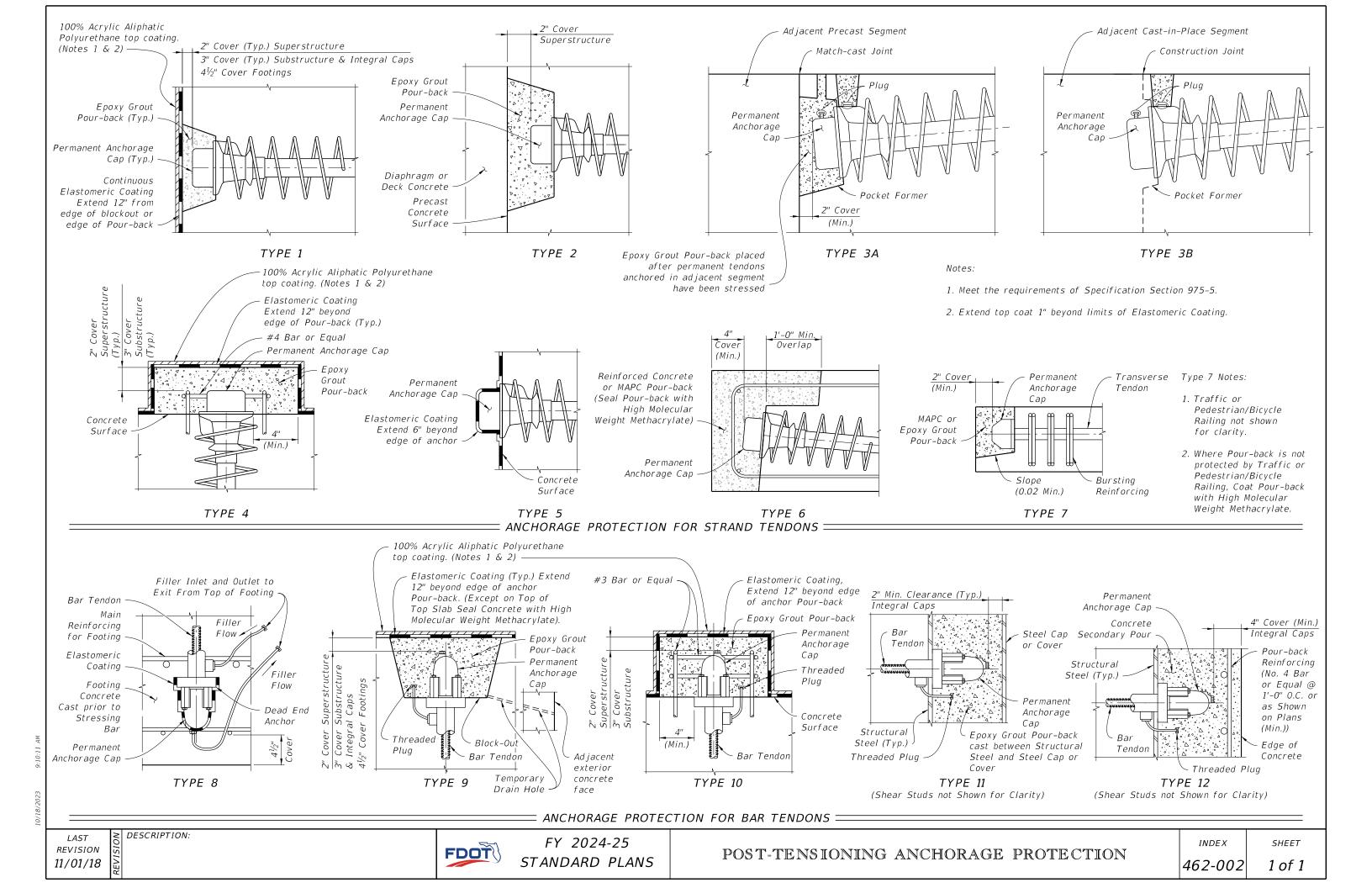
SHEET 1 of 3

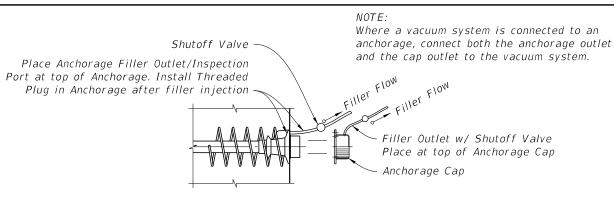




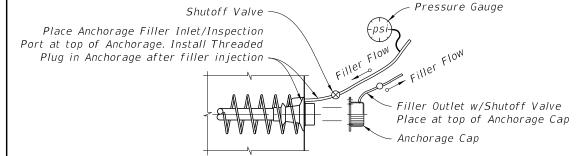




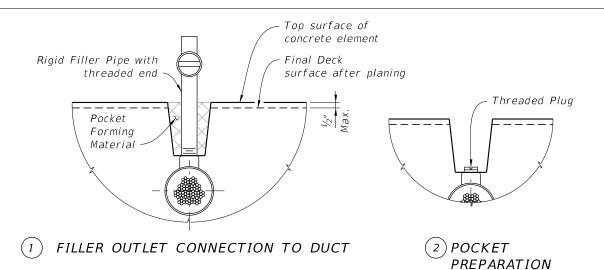


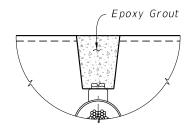


FACE INSPECTED ANCHORAGE WITH FILLER OUTLET



FACE INSPECTED ANCHORAGE WITH FILLER INLET





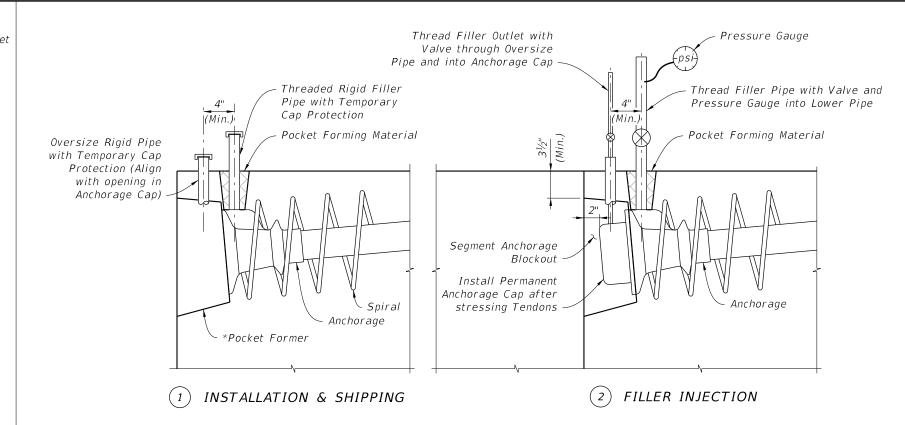
PROCEDURE:

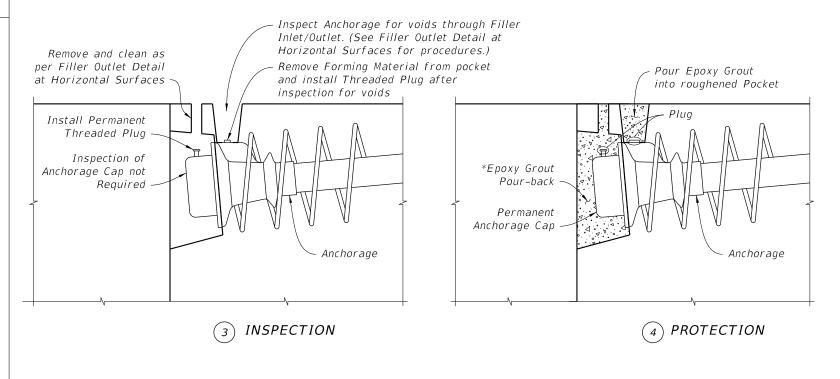
- 1. After filler injection is completed, Remove Pocket Forming Material and Rigid Filler Pipe.
- 2. Inspect Tendon for voids as necessary.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Clean threads and rethread as required.
- 5. Install Threaded Plug into Outlet to form a tight fit.
- 6. Clean and roughen sides of pocket.
- 7. Fill Pocket with Epoxy Grout.

(3) FILLING POCKET

DESCRIPTION:

= FILLER OUTLET DETAIL AT HORIZONTAL SURFACES =





NOTES:

- 1. Holes used for the Inspection and Filler Inlets/Outlets may be formed using tapered pipes or mandrels.
- 2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.
- * Round () Pocket Former Gravity fed placement of epoxy grout acceptable Modified Square Pocket Former - Gravity fed placement of epoxy grout acceptable Square Pocket Former - Vacuum epoxy grouting required

REVISION 11/01/18



POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

TOP INSPECTED ANCHORAGE WITH FILLER

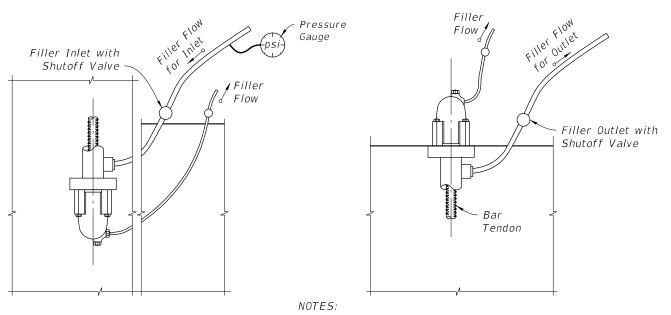
INLET INSTALLATION, FILLER INJECTION,

INSPECTION & PROTECTION

INDEX

SHEET

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INLET END

(EMBEDDED ANCHORAGE SHOWN; ANCHORAGE AT CONCRETE SURFACE SIMILAR)

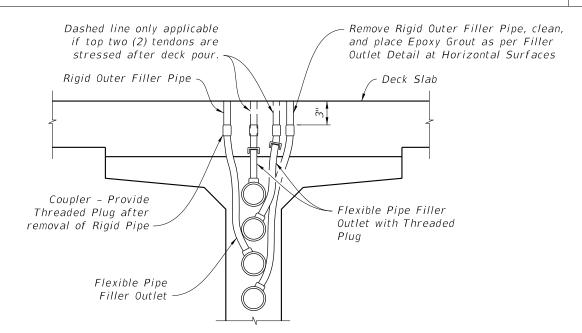
1. Anchor or Nut to allow for flow of Filler into Cap.

2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.

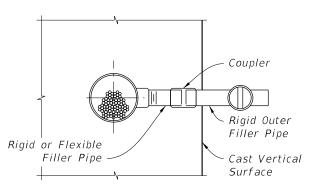
OUTLET END

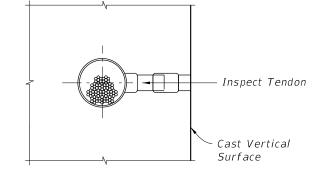
FILLER INLET AND OUTLET DETAILS FOR BAR TENDONS

(VERTICALLY ORIENTED TENDON SHOWN; HORIZONTALLY ORIENTED TENDON SIMILAR)

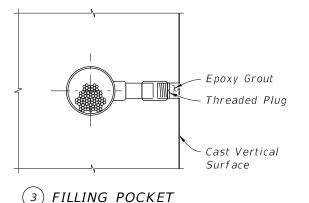


TENDONS AT HIGH POINTS AND 3' FROM HIGH POINTS (FILLER OUTLET)





 $(\ {\scriptscriptstyle 1}\)$ FILLER OUTLET CONNECTION TO TENDON

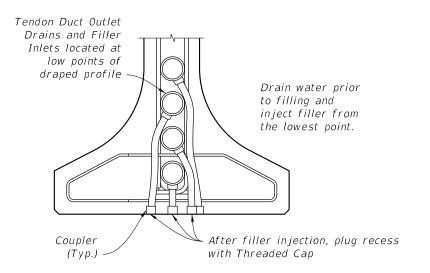


(2) POCKET PREPARATION

PROCEDURE:

- 1. Remove Rigid Filler Pipe or drill Grout in flexible pipe.
- 2. Inspect tendon for voids.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Install Threaded Plug into Outlet to form a tight fit.
- 5. Over-ream hole (${}^{1}\!\!/_{4}$ " Ø over-ream). Clean and roughen sides.
- 6. Fill pocket with epoxy grout.

FILLER OUTLET DETAIL AT VERTICAL SURFACES



TENDONS AT LOW POINTS (FILLER INLET / DRAIN)

FILLER INLET AND OUTLET DETAILS FOR I-GIRDERS

DETAILS FOR C.I.P. BOXES WITH INTERNAL TENDONS SIMILAR. WEB REINFORCING NOT SHOWN FOR CLARITY.

REVISION 11/01/16

DESCRIPTION:

FDOT

FY 2024-25 STANDARD PLANS

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

INDEX

SHEET

462-003

- U.S. COAST GUARD NOTIFICATION: Notify the local office of the U.S. Coast Guard at least 30 days prior to beginning of construction of the Fender System.
- 14" SQUARE PRESTRESSED CONCRETE PILES Provide 14" Square Prestressed Concrete Piles of sufficient length to achieve a minimum embedment of 20' into soil having a blow count greater than or equal to 6 ($N \ge 6$). Pile splices and build-ups are not permitted. Use only 14" Square Prestressed Concrete Piles with 8 $\frac{1}{2}$ " diameter Low Relaxation Strands fabricated in accordance with Index 455–014.
- PLASTIC LUMBER AND STRUCTURAL COMPOSITE LUMBER WALES: Provide only Plastic Lumber (Thermoplastic Structural Shapes) and Structural Composite Lumber (Reinforced Thermoplastic Structural Shapes) Wales in accordance with Specification Section 973. Wales shall be continuous and spliced only at locations shown on the plans.
- PLASTIC LUMBER DECKING FOR CATWALKS: Provide Plastic Lumber decking for catwalks when called for in the Plans in accordance with Specification Section 973.

Install Plastic Lumber Decking according to manufacturer's recommendations using stainless steel $\#10 \times 3$ " (minimum) deck screws.

FIBERGLASS OPEN GRATING FOR CATWALKS: Provide Fiberglass Open Grating for catwalks when called for in the Plans. Fiberglass Open Grating shall be a heavy duty design suitable for exterior installations. Maximum gap opening on the walkway surface shall be $1\frac{1}{2}$ ". Design live loads and deflections shall be a 50 psf uniformly distributed load with a maximum deflection of $\frac{3}{6}$ " or L/120 at the center of a simple span and a concentrated load of 250 pounds with a maximum deflection of $\frac{1}{4}$ " at the center of a simple span. Color of Fiberglass Open Grating shall be gray or black.

Install Fiberglass Open Grating according to manufacturer's recommendations using stainless steel hardware, screws, bolts, nuts and washers. Attach Fiberglass Open Grating to Wales and Deck Supports at a 2'-0" maximum spacing so as to resist pedestrian live loads and uplift forces from wind, buoyancy and wave action.

- CLEARANCE GAUGE AND LIGHT: Clearance Gauge to be furnished and installed by the Contractor. Clearance Gauge width and numeral height is dependent on visibility distance. The required visibility distance shall be determined by the United States Coast Guard District Commander. Provide and install Clearance Gauge Light in accordance with Specification Section 510 and Index 510-001.
- NAVIGATION LIGHTS: Provide and install Navigation Lights in accordance with Specification Section 510, Index 510-001 and/or project specific details. Provide and maintain Temporary Navigation Lights during construction until permanent Navigation Lights are operational.
- BOLTS, THREADED BARS, NUTS, SCREWS AND WASHERS: Furnish stainless steel Bolts in accordance with ASTM F593 Type 316. Furnish stainless steel Threaded Bars in accordance with ASTM A193 Grade B8M. Furnish stainless steel Nuts in accordance with ASTM F594 Type 316. Furnish stainless steel Screws in accordance with ASTM F593 Type 305. Furnish stainless steel Washers compatible with Bolts, Threaded Rods and Nuts under heads and nuts. Torque Nuts on 1" diameter Bolts and Threaded Bars to 150 lb-ft. Keep threads on Bolts, Threaded Bars and Nuts free from dirt, coarse grime and sand to prevent galling and seizing during tightening.

SPLICE PLATES: Furnish Splice Plates in accordance with ASTM A240 Type 316.

WIRE ROPE: Provide wire rope meeting one of the following requirements:

- 1. $\frac{1}{2}$ " diameter 6x19, 6x25 or 6x37 class IWRC Type 316 stainless steel wire rope with a minimum breaking strength of 18,000 lbs.
- 2. ½" diameter 6x19 galvanized wire rope with ultraviolet ray resistant polypropylene impregnation having an outside diameter of 5/8" with a minimum breaking strength of 22,000 lbs. Protect all ends with heat shrinkable end caps compatible with the rope's polypropylene that provide an effective water-tight seal.

FENDER SYSTEM ENERGY CAPACITY: Energy Capacity = 38 ft-k

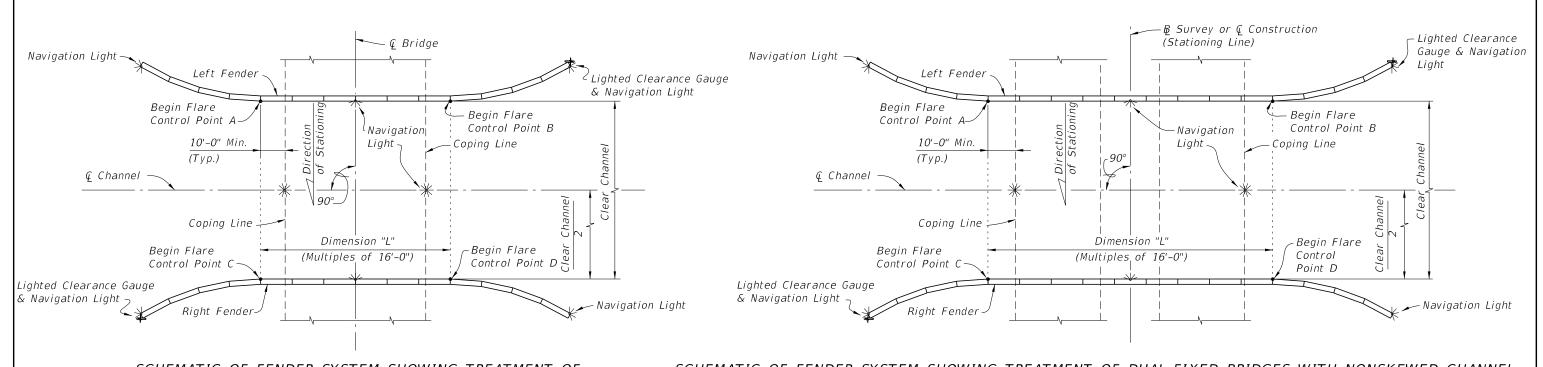
GENERAL NOTES

10/14/2023

LAST REVISION 07/01/14

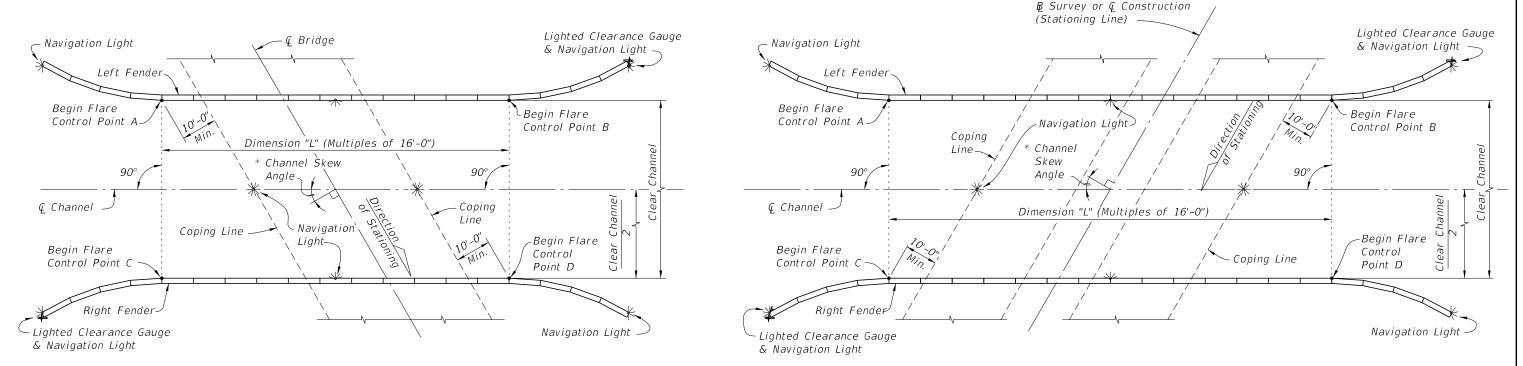
DESCRIPTION:





SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH NONSKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH NONSKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH SKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH SKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)

* See Structures Plans, Plan and Elevation and Foundation Layout Sheets for magnitude and orientation of Channel Skew Angle.

CROSS REFERENCES:

For Stations and Offsets of referenced Control Points A, B, C and D, Dimension "L" and Clear Channel Width see Fender System Table of Variables in Structures Plans. For Navigation Light Details see Design Standards Index 510-001.

LAYOUT GEOMETRY

REVISION 07/01/11

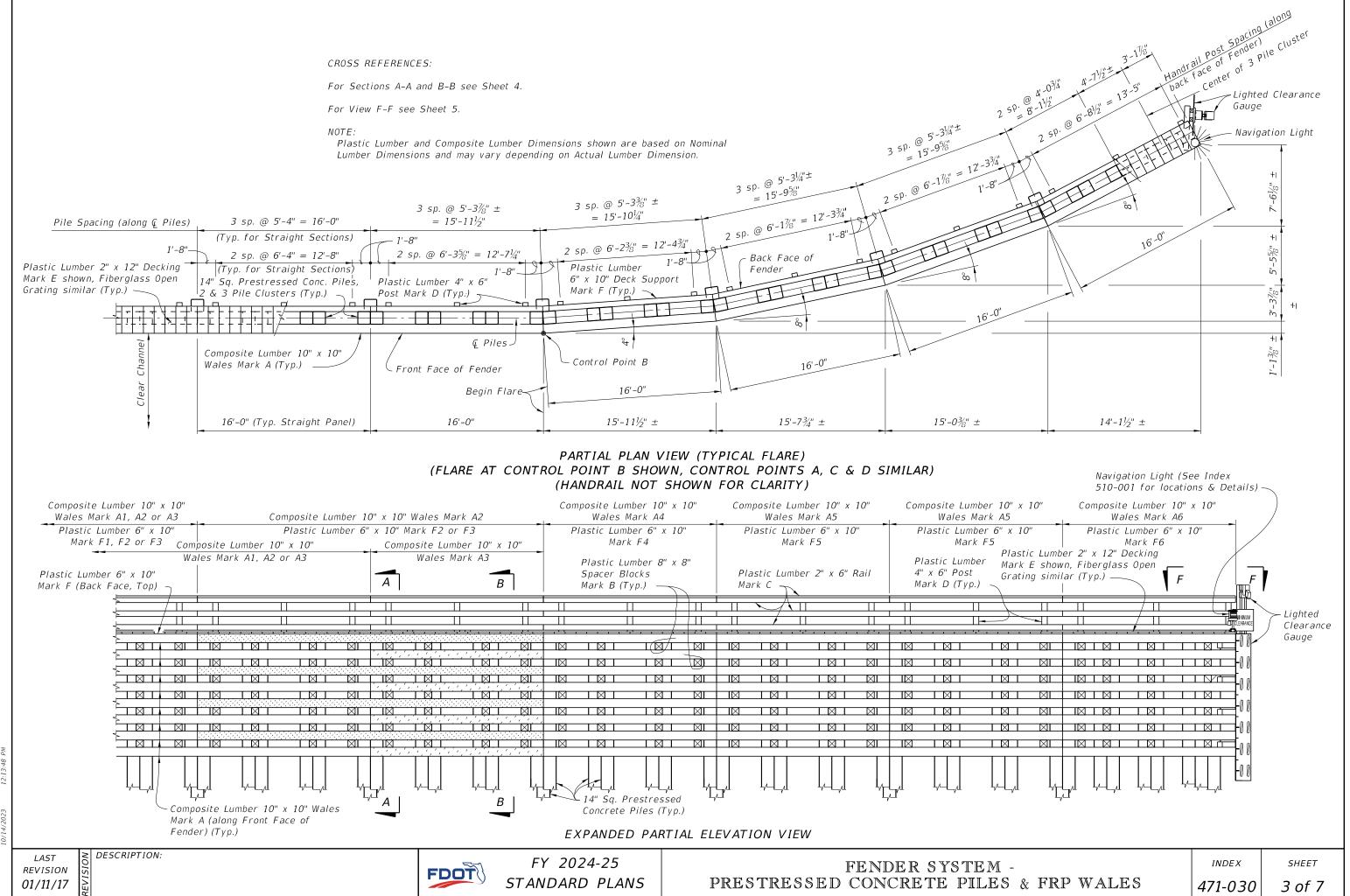
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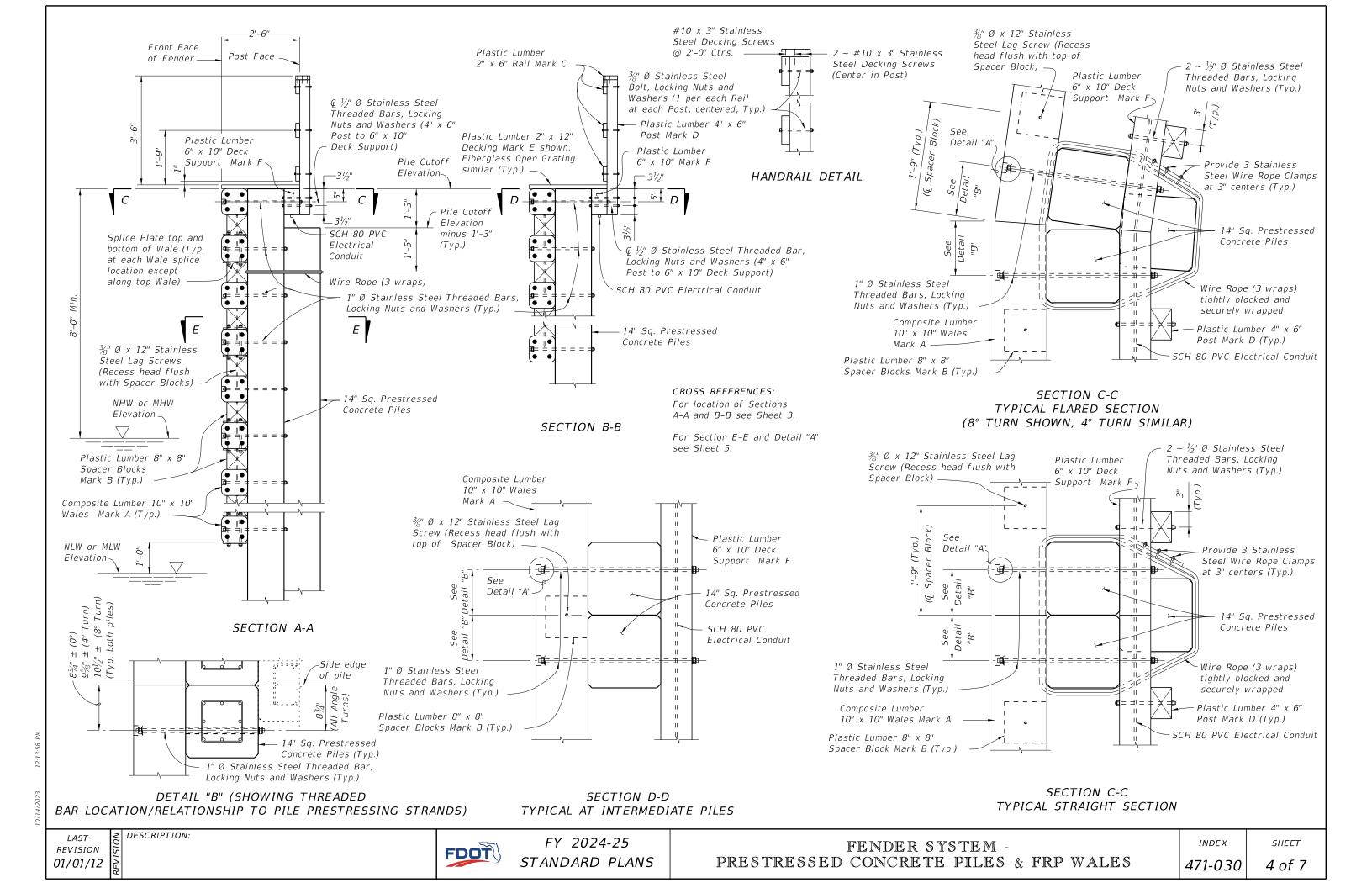
FY 2024-25 STANDARD PLANS

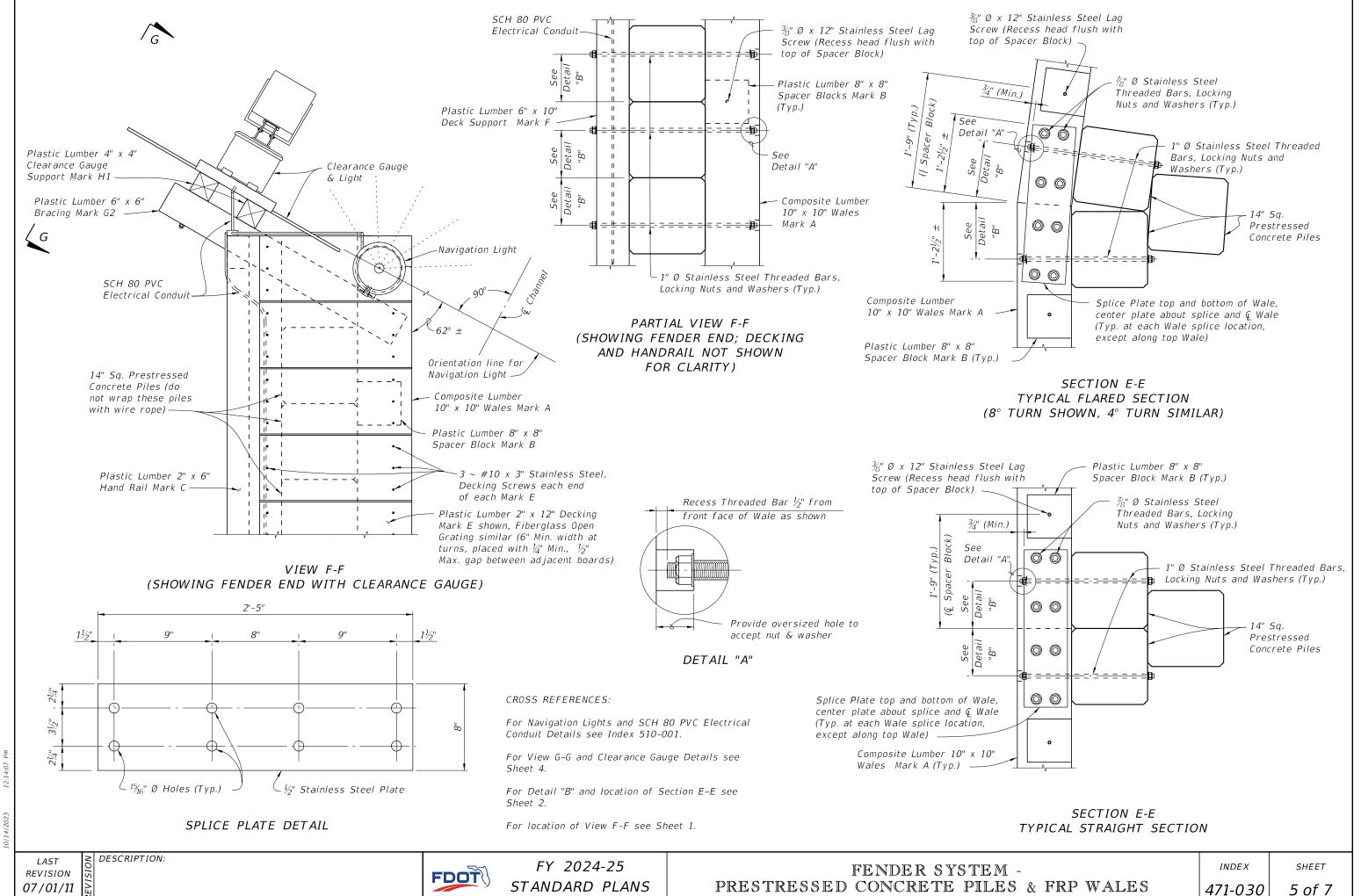
FENDER SYSTEM -PRESTRESSED CONCRETE PILES & FRP WALES INDEX

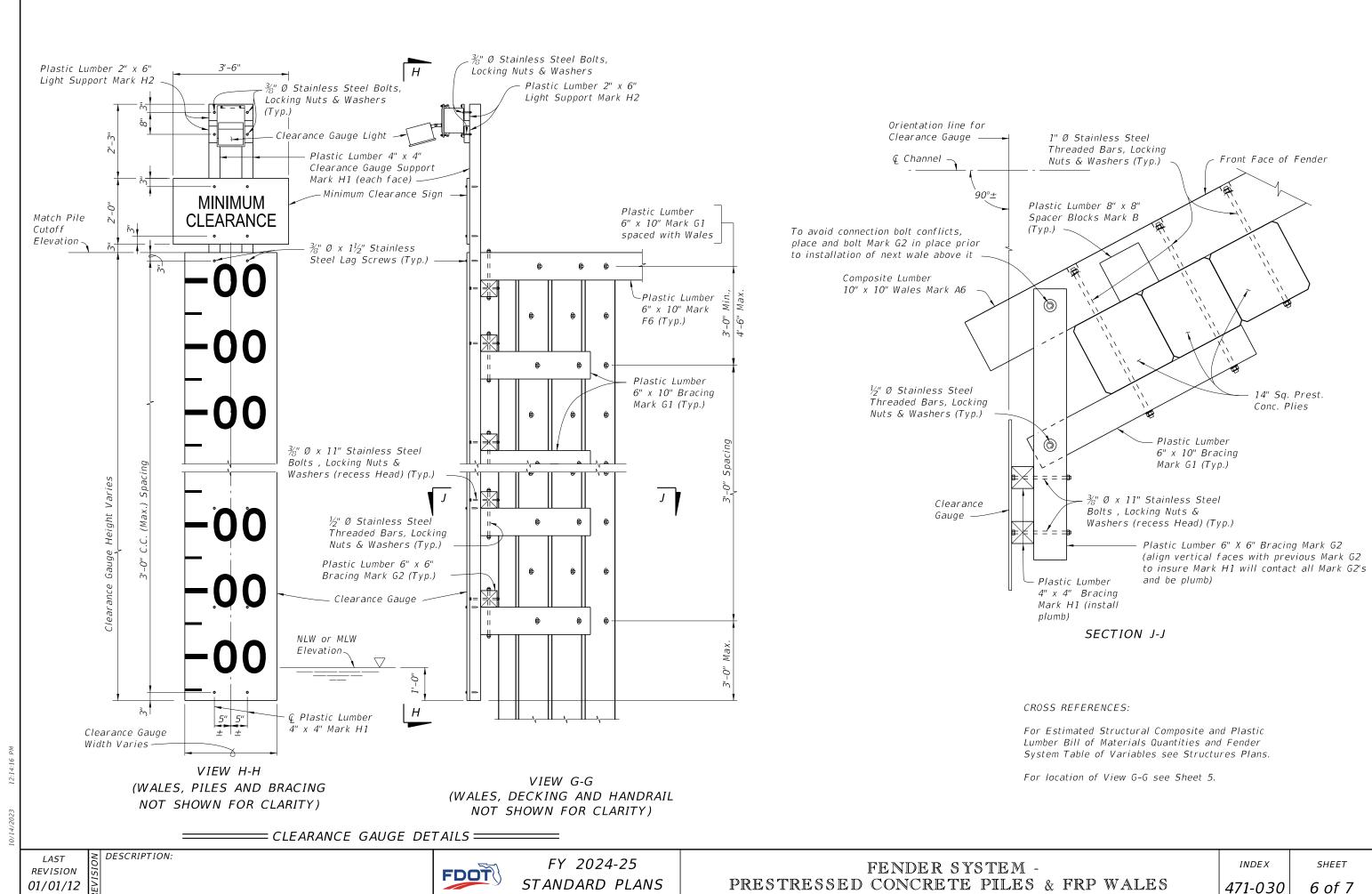
SHEET 2 of 7

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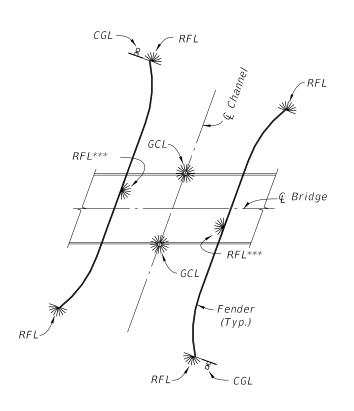
* STRUCTURAL COMPOSITE LUMBER BILL OF MATERIALS					
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY
A1	10" X 10" COMPOSITE LUMBER	32'-0" (STRAIGHT)	266.6	nber	
A2	10" X 10" COMPOSITE LUMBER	32'-0"	266.6	Plastic Lur	res Plans
A3	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	Structural Composite and Plastic Lumber	e in Structures
A4	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	tructural Co	Materials Table in
A5	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	~	Bill of Ma
A6	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	See	

*	All Plastic Lumber and Composite Lumber Dimensions and Quantities shown
	are based on Nominal Lumber Dimensions and may vary depending on Actual
	Lumber Dimension.

^{**} Provide Fiberglass Open Grating in lieu of 2" X 12" Plastic Lumber when called for in the Plans. Mounting hardware shall be Stainless Steel, install per Manufacturer's recommendations. See Structures Plans for Notes and Details.

	*	PLASTIC LUMBER BILL OF MA	TERIALS		
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY
В	8" X 8" PLASTIC LUMBER	8" (STRAIGHT)	3.6		
С	2" X 6" PLASTIC LUMBER	16'-0" (STRAIGHT) (Trim & Miter Ends as required)	16.0		
D	4" X 6" PLASTIC LUMBER	4'-4" (STRAIGHT)	8.7		
** E	2" X 12" PLASTIC LUMBER	2'-6" (STRAIGHT) (Miter as required, 6" Min. width)	5.0		
F 1	6" X 10" PLASTIC LUMBER	32'-0" (STRAIGHT)	160.0	mber	
F2	6" X 10" PLASTIC LUMBER	31'-11"	159.6	Estimated Structural Composite and Plastic Lumber	ıres Plans
F3	6" X 10" PLASTIC LUMBER	15'-11"	79.6	omposite an	le in Structu
F4	6" X 10" PLASTIC LUMBER	15'-91/4"	78.8	stimated Structural Composite and Plastic L Bill of Materials Table in Structures Plans	
F5	6" X 10" PLASTIC LUMBER	15'-81/4"	78.4	Estimated S Bill of Ma	
F6	6" X 10" PLASTIC LUMBER	15'-101/4"	79.3	See	
G 1	6" X 10" PLASTIC LUMBER	3'-8" (STRAIGHT)	18.3		
G2	6" X 6" PLASTIC LUMBER	4'-1" (STRAIGHT)	12.3		
H1	4" X 4" PLASTIC LUMBER				
H2	2" X 6" PLASTIC LUMBER	1'-2" (STRAIGHT)	1.2		

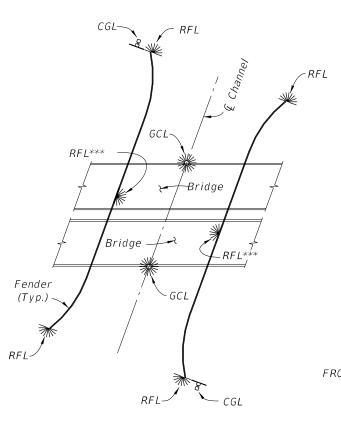
≥ DESCRIPTION:



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITH FENDERS

GCL-

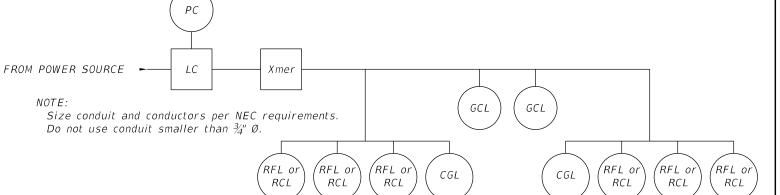
-∕Bridge



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR DUAL BRIDGES WITH FENDERS

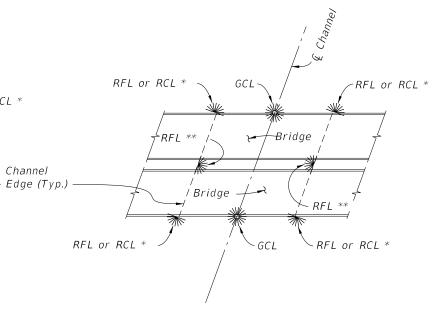
NAVIGATION LIGHT NOTES:

1. Provide Navigation Light System in compliance with Specifications Section 510.



TYPICAL ELECTRICAL SCHEMATIC DIAGRAM

POWER CONDUCTORS			
DISTANCE (feet)	VOLTS	CONDUCTOR	TRANSFORMER
0 - 75	120	#12 AWG	N/A
75 - 500	120 or 240	#10 AWG	N/A
500-1000	240	#10 AWG	N/A
1000-2000	480	#10 AWG	2 KVA
2000-5000	480	#8 AWG	2 KVA
5000-10000	480	#6 AWG	2 KVA
over 10000	480	#4 AWG	2 KVA



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITHOUT FENDERS

NAVIGATION LIGHT SYSTEM SCHEMATIC FOR DUAL BRIDGES WITHOUT FENDERS

- * Use RFL when Pier is at Channel Edge and see CFR, Title 33, part 118 for Mounting Height restrictions. Use RCL otherwise.
- ** Mounted only on the Pier that defines CM, otherwise does not apply.

RFL **

RFL or RCL *

*** RFL to be located at mid length of straight portion of fender.

LEGEND

SYMBOL	DESCRIPTION

Lighting Contactor

Photocell Control

Transformer (If Required)

RFL Red Pier/Fender Light (180° visibility) or RCLRed Channel Margin Light (180° visibility)

Green Center Channel Light (360° visibility)

CGLClearance Gauge Light

> Channel Margin or Pier inner surface whichever defines Channel Edge.

REVISION 11/01/17

FDOT

FY 2024-25 STANDARD PLANS

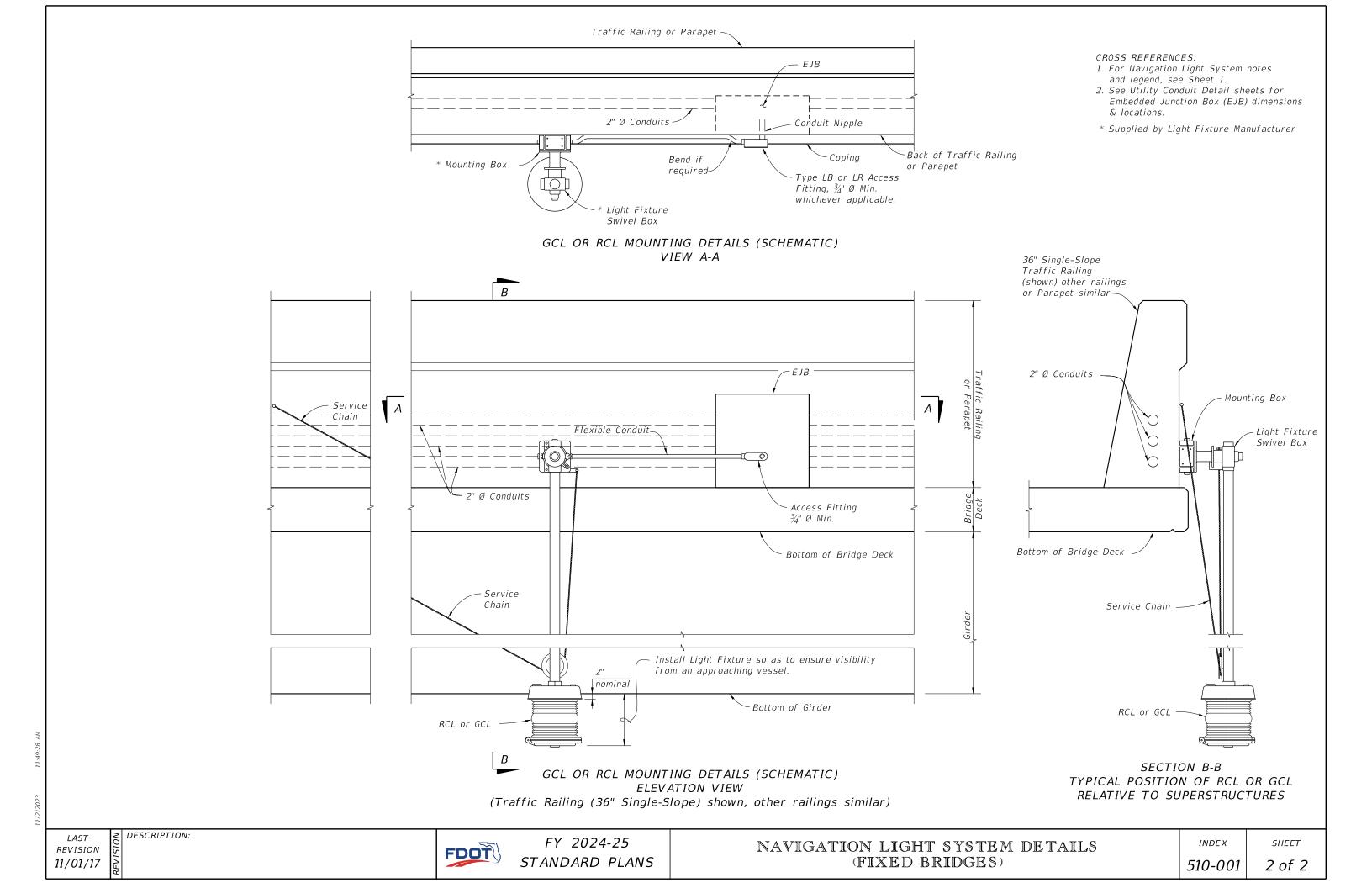
510-001 1 of 2

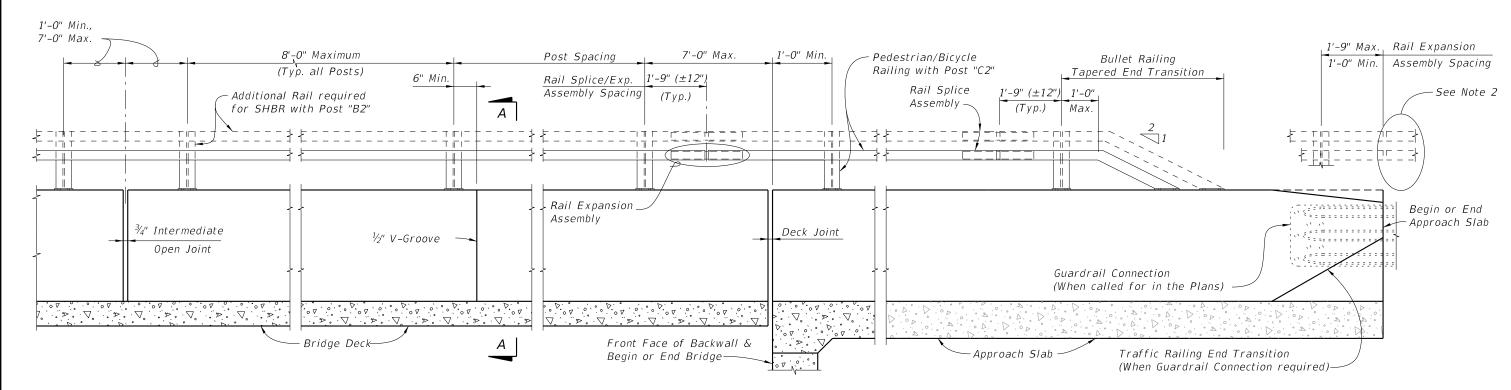
DESCRIPTION:

RFL or RCL *

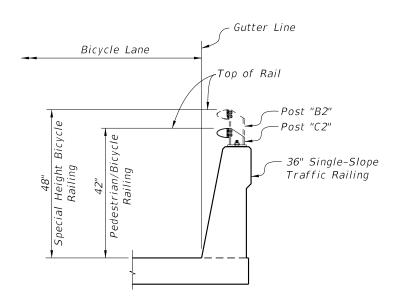
RFL or RCL *

(FIXED BRIDGES)





ELEVATION OF INSIDE FACE OF TRAFFIC RAILING WITH PEDESTRIAN/BICYCLE BULLET RAILING



SECTION A-A TYPICAL SECTION THRU BRIDGE DECK (APPROACH SLAB SIMILAR)

NOTES:

- 1. A Bullet Railing Tapered-End Transition is required for all approach ends of Bullet Railings on Traffic Railings. When Guardrail Connection is required terminate the Bullet Railing Tapered-End Transition at beginning of the Traffic Railing End Transition.
- 2. Where Bullet Railing continues on retaining wall mounted Traffic Railings or Barriers, provide a Bullet Railing Tapered End Transition at the terminus of the Bullet Railing.

CROSS REFERENCES:

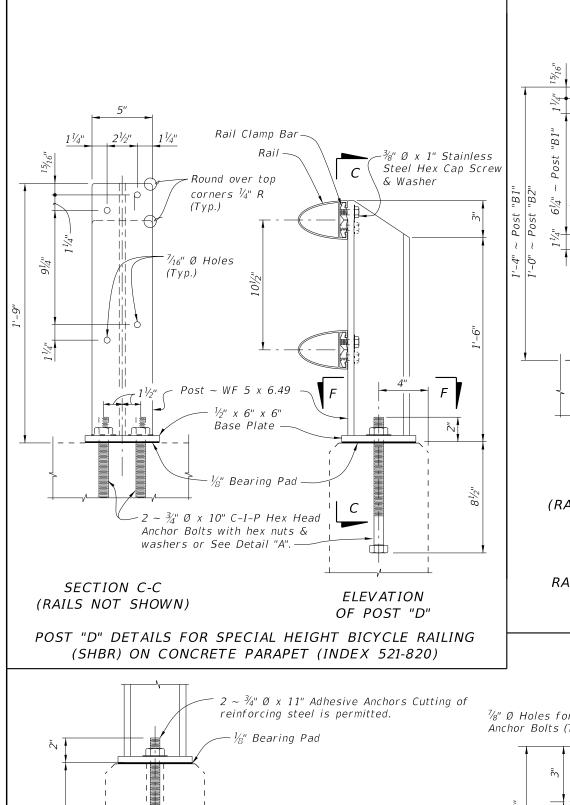
Work in conjunction with Index 515-022.

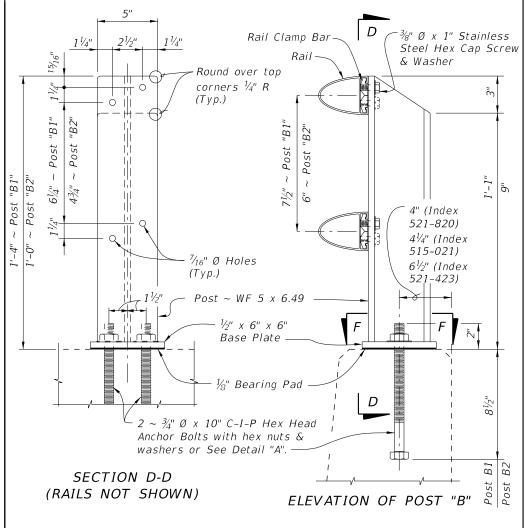
For Traffic Railing Details, Reinforcement and Notes see Index 521-427.

10/14/2023 12:15

LAST OF DESCRIPTION:
REVISION 11/01/17



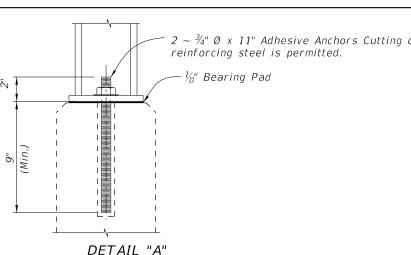




POST "B1" DETAILS FOR SHBR ON TRAFFIC RAILING (INDEX 521-423) AND FOR PEDESTRIAN/BICYCLE RAILING (PBR) ON CONCRETE PARAPETS (INDEX 521-820) POST "B2" DETAILS FOR SHBR ON TRAFFIC RAILING (INDEX 521-427 AND 515-021)

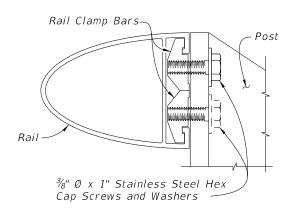
4½" (Index 515-021) 6½" (Index 521-423) Ε 11/4" Rail Clamp Bar ¾" Ø x 1" Stainless Steel Hex Cap Screw 7∕₁₆" Ø Holes & Washer (Typ.)Round over top corners 1/4" R (Typ.)Post ~ WF 5 x 6.49 ½" x 6" x 6" Base Plate Post Post $rac{1}{8}$ " Bearing Pad Ε $2 \sim \frac{3}{4}$ " Ø x 10" C-I-P Hex Head Anchor Bolts with hex nuts & washers or See Detail "A". _ Face of Traffic Railing SECTION E-E **ELEVATION** OF POST "C" (RAIL NOT SHOWN)

POST "C1" DETAILS FOR PEDESTRIAN/BICYCLE RAILING (PBR) ON TRAFFIC RAILINGS (INDEX 521-423) POST "C2" DETAILS FOR PBR ON TRAFFIC RAILING (INDEX 521-427 & 515-021)



6" ⁷⁄₈" Ø Holes for Anchor Bolts (Typ.) H-Beam Post ~ WF 5 x 6.49

SECTION F-F BASE PLATE DETAIL



RAIL TO POST CONNECTION DETAIL

CROSS REFERENCES:

For post spacing on Concrete Parapets see Index 521-820.

For post spacing on Traffic Railings see Index 515-021.

For Rail Details see Sheet 2.

For Railing Notes and Tapered End Transition Details see Sheet 3.

DESCRIPTION: REVISION 11/01/17

ALTERNATE ANCHOR BOLT

(Concrete Parapet Shown, Traffic Railings Similar)

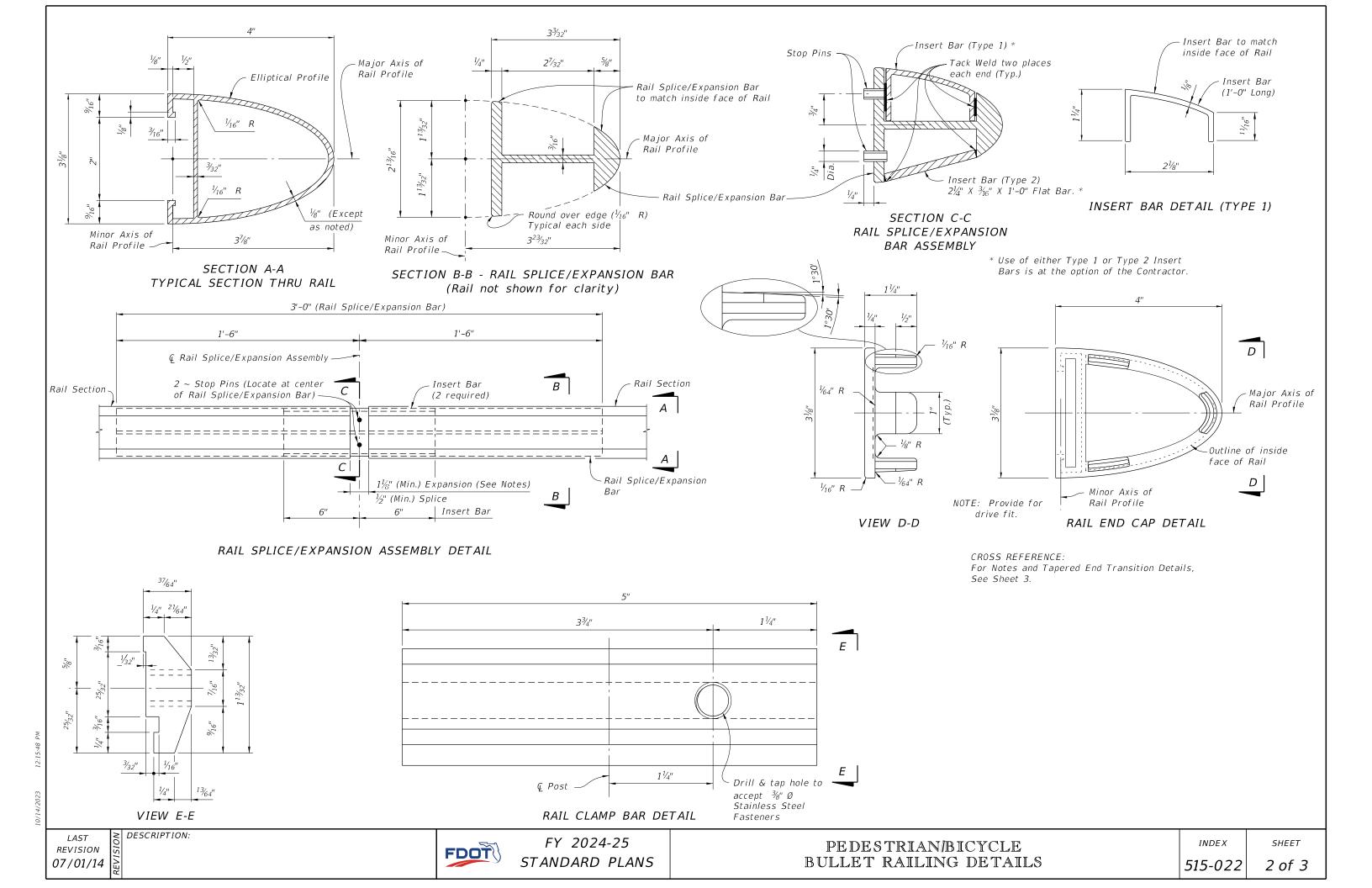
FDOT

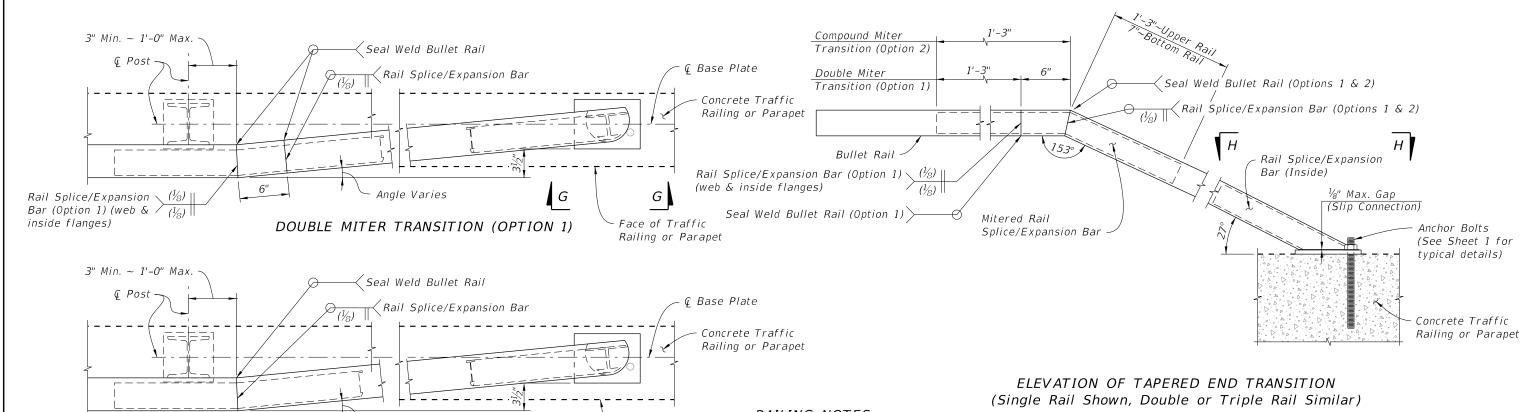
FY 2024-25 STANDARD PLANS

PEDESTRIAN/BICYCLE BULLET RAILING DETAILS INDEX

SHEET

1 of 3





PARTIAL PLAN OF TAPERED END TRANSITIONS

COMPOUND MITER TRANSITION (OPTION 2)

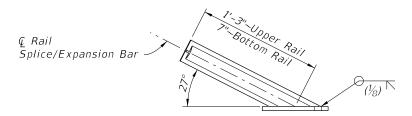
Angle Varies

G

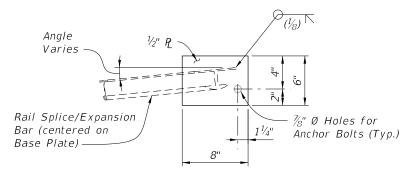
Face of Traffic

Railing or Parapet

(Single Rail Shown, Double or Triple Rail Similar)



VIEW G-G TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)



VIEW H-H TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)

RAILING NOTES:

- Work this Index with Index 521-423, 521-427, 521-428, 521-820 and 515-021 and Specification Section 515.
- Shop Drawings: Submit shop drawings prior to fabrication. A. Include post and rail splice/expansion assembly location for curved alignments with radii < 40 feet and for all end terminations.
- - A. Supply Aluminum materials In accordance with Specification Section 965 and the following: Wrought Aluminum Post: ASTM B221, Alloy 6061-T6 or 6351-T5 Rail End Cap: ASTM B26 sand cast aluminum alloy 356.0-F Plate and Bars: ASTM B209 Alloy 6061-T6 Rails: ASTM B221 Alloy 6061-T6 or 6351-T5. Stop Pins: Press-fit aluminum or stainless steel pins or tubes
 - B. Stainless Steel Fasteners: ASTM F-593, Alloy Group 2 (316).
 - C. Bearing Pads: Plain or Fiber Reinforced meeting Specification Section 932 for Ancillary Structures.
 - D. Anchor Bolts: Galvanized ASTM A307 Grade 36 Hex Head. Galvanized ASTM 1554 Grade 55 Threaded rods for Adhesive Anchors.
- 4. Layout.
 - A. Posts shall be uniformly spaced with reasonable consistency.
 - B. Tapered End Transitions are required at the terminus of the approach ends of Bullet Railing mounted on a Traffic Railing. Bullet Railings on concrete parapets shielded by a traffic railing do not require Tapered End Transitions unless noted otherwise in the Plans.
 - C. Adjust post spacing's to avoid parapet obstacles, such as armor expansion plates, by 9 inches minimum.
 - D. Rails shall be continuous over a minimum of 3 posts, except that lengths less than 12 feet need only be continuous over 2 posts.
 - E. Space splices at 40 feet maximum. Splice all rails in a given railing section at about the same center line.
 - F. Provide rail expansion assemblies in panels between posts on either side of a bridge expansion joint. Rail expansion assemblies are similar to the rail splice assemblies with increased space at the expansion assembly to allow for movement equal to 1.5 times the bridge joint opening or 1" greater than the expected joint movement.
- - A. Set rails near bridge expansion joints to allow for expected movement.
 - B. Cutting of reinforcing steel is permitted for post installed anchors.
- 6. Payment: Includes the full cost of installed bullet railing. Cost of the Concrete Parapet or Traffic Railing is separate.

REVISION 11/01/22

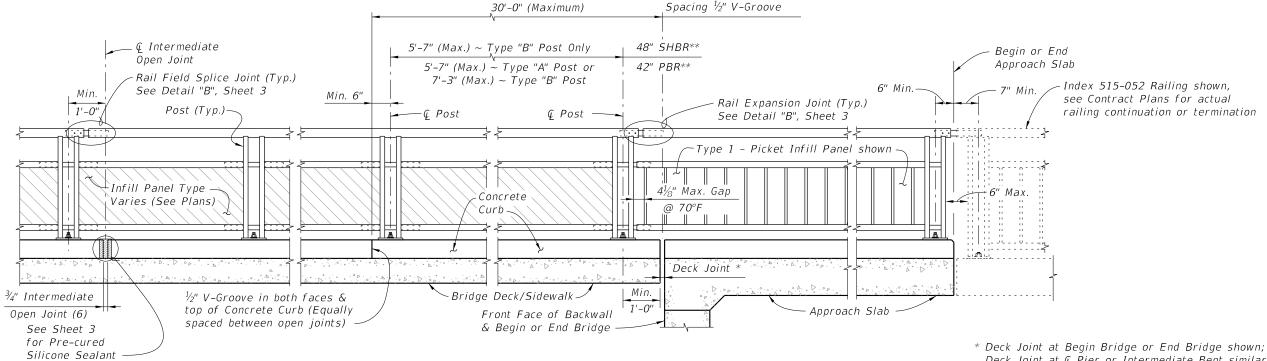
FDOT

FY 2024-25 STANDARD PLANS

PEDESTRIAN/BICYCLE BULLET RAILING DETAILS INDEX

SHEET

515-022 3 of 3 (Scheme 2 shown, other Schemes similar, Reinforcing Steel not shown for clarity)



ELEVATION OF INSIDE FACE OF RAILING (Scheme 2 shown with Post "A", other Schemes similar, Reinforcing Steel not shown for clarity)

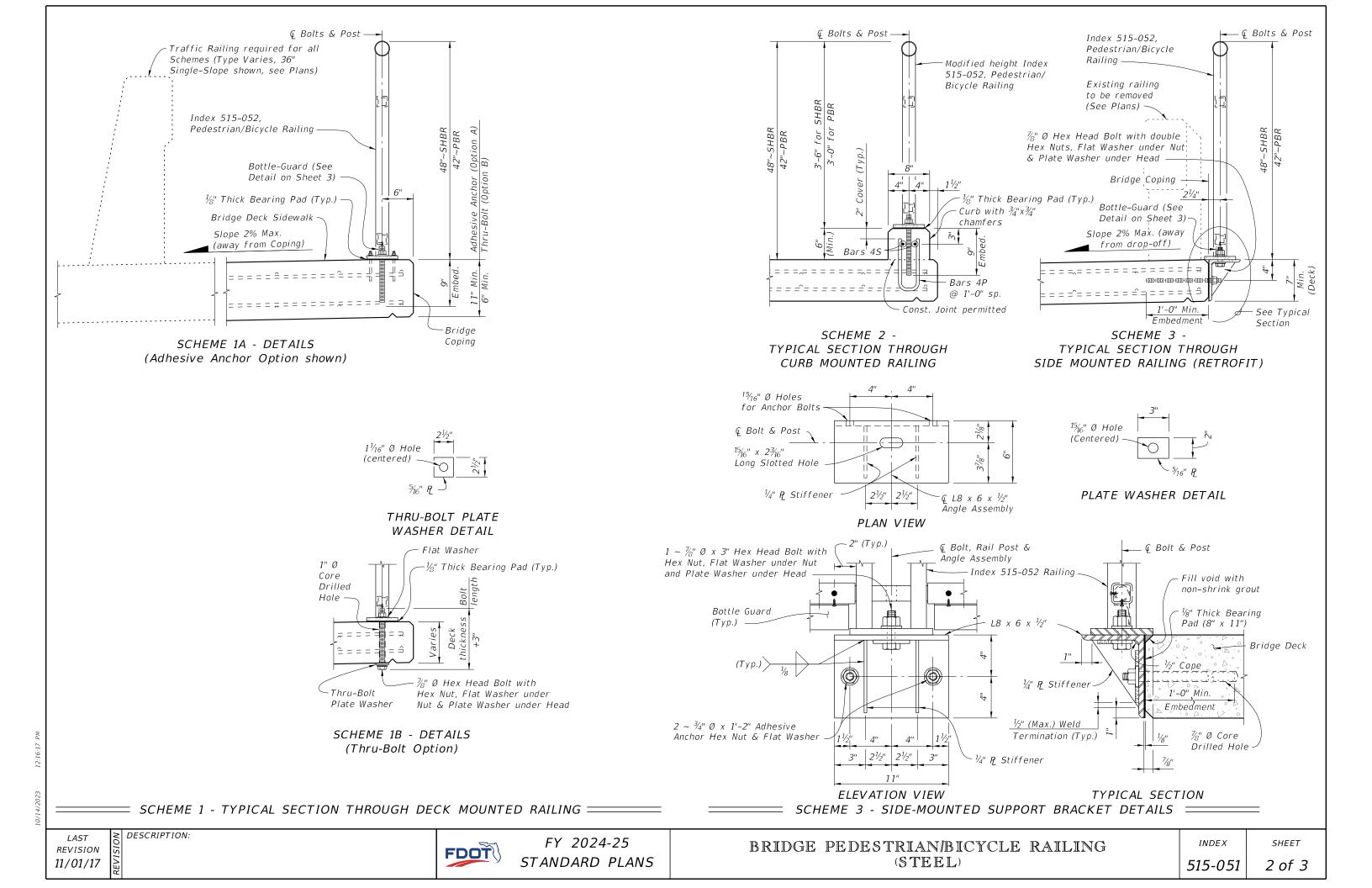
- 1. Shop Drawings are required.
- 2. Work this Index with Index 515-052 Bicycle/Pedestrian Railing Details (Steel) and Specification Section 515. Refer to the SPI for Design Criteria and Limits of Use.
- 3. Materials:
 - A. Steel: Galvanized after fabrication
 - a. Fasteners: Hex Head Bolt ASTM A307, Hex Nuts ASTM A563, Washers ASTM F436
 - b. Support Bracket (Scheme 3) L-shape and Stiffener Plate: ASTM A36
 - c. Bottle-guard (Schemes 1 & 3) L-shape: ASTM A36
 - B. Concrete: Same as bridge deck
 - C. Pre-cured Silicone Sealant: Specification Section 932
 - D. Bearing Pads: Provide \(\frac{1}{18} \)" Plain, Fabric Reinforced or Fabric Laminated bearing pads that meet the requirements of Specification Section 932 for Ancillary Structures.
- 4. See Structures Plans, Superstructure Sheets for bridge information including concrete type, deck expansion joint locations and orientations, and thermal movement.
- 5. Railings:
 - A. For thermal movement greater than 4" (up to a maximum of 5"), clear opening between adjacent pickets, or panels at Rail Expansion Joints above Deck Joints must be reduced to $3\frac{1}{3}$ ".
 - B. For treatment of railings on skewed bridges see Index 521-427.
- 6. Curbs:
 - A. Match open curb joints at Deck Expansion Joint locations to the deck joint dimension.
 - B. Construct Concrete Curb (Scheme 2) vertical with the top surface finished level transversely. See Concrete Curb Details Sheet 3.
 - C. Provide $\frac{3}{4}$ " Intermediate open joints in curbs coinciding with the $\frac{3}{4}$ " joints in the traffic railing.
- 7. Payment: Support bracket (Scheme 3) is incidental to the cost of railing. Curb concrete and reinforcing steel (Scheme 2) are included in the bridge deck quantities.

Deck Joint at Q Pier or Intermediate Bent similar.

** SHBR~Special Height Bicvcle Rail PBR~Pedestrian/Bicycle Rail

REVISION 11/01/17 DESCRIPTION:

FDOT



Top Rail or

— Steel Sleeve:

Round over both ends 1" NPS (Sch. 40) for handrails

Handrail

6" Max. @ maximum movement

Deck Expansion Joint

Field Splice Slip Joint

6" Min.

Posts Pickets (Typ.) Intermediate or Bottom Rail section

 $6\frac{3}{4}$ "

2.50 OD x 0.125 Wall for top rail SQUARE RAILS - INTERMEDIATE OR BOTTOM RAIL

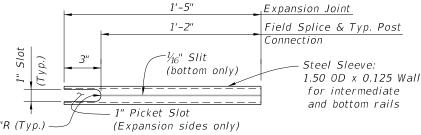
3" Min.

-3" Min.

ROUND RAILS - TOP RAIL OR HANDRAIL

- * $\frac{1}{4}$ " Ø x $\frac{3}{4}$ " Pan Head Stainless Steel (Type 316 or 18–8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A $\frac{3}{4}$ Ø plug weld may be substituted for the two set screws at expansion joints.
- ** Embedded length may be 4" for plug welded connection. *** Increase handrail sleeve embedment to 8" for Expansion Joint openings
- greater than 2". **** Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".

of rails to remove sharp edges (Typ.)



INTERMEDIATE OR BOTTOM RAIL - STEEL SLEEVE DETAIL (Bottom Side Shown)

CONVENTIONAL REINFORCING

SIZE

LENGTH

2'-0"

As Regd.

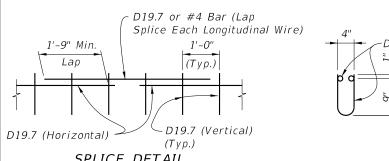
As Regd.

BAR 4S

== DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =

ALTERNATE REINFORCING (WWR) DETAILS

NOTE: Place wire panels to minimize the end overhang. End Overhangs greater than $4\frac{3}{4}$ " are not permitted.



SPLICE DETAIL (Between WWR Sections)

STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL MARK -D19.7 (Typ.) S

WWR SECTION DETAIL

CURB REINFORCING STEEL NOTES:

ESTIMATED CONCRETE CURB

QUANTITIES (SCHEME 2)

UNIT

CY/LF

LB/LF

ITEM

Reinforcing Steel

Concrete

DESCRIPTION:

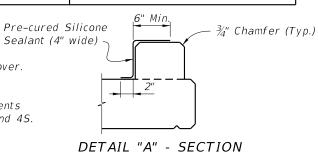
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8"

QUANTITY

0.0124

4.01

5. Deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931 may be used in lieu of all Bars 4P and 4S.

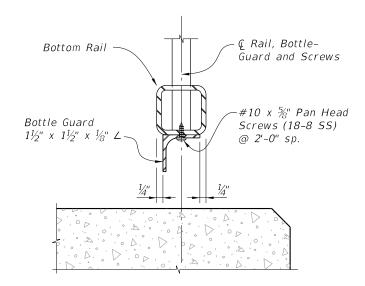


AT INTERMEDIATE OPEN JOINT INTERMEDIATE JOINT SEAL NOTE:

At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

BAR 4P

SCHEME 2 - CONCRETE CURB DETAILS

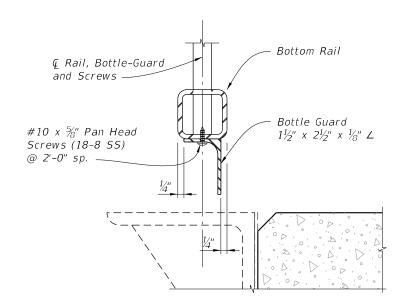


TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

== SCHEME 1 - BOTTLE GUARD DETAIL ====

CROSS REFERENCE:

See Sheet 1 for Bridge Railing Notes.



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL =

REVISION 11/01/16

FDOT

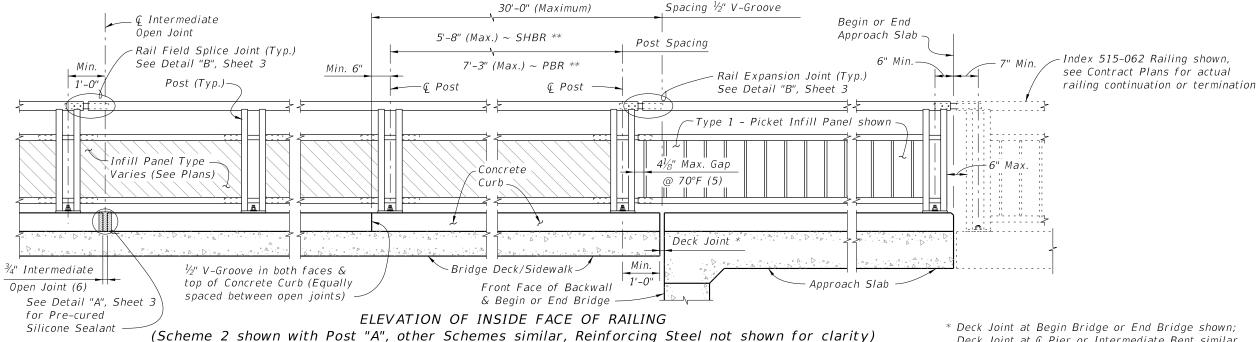
FY 2024-25 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING (STEEL)

INDEX

SHEET

515-051 3 of 3 (Scheme 2 shown, other Schemes similar, Reinforcing Steel not shown for clarity)



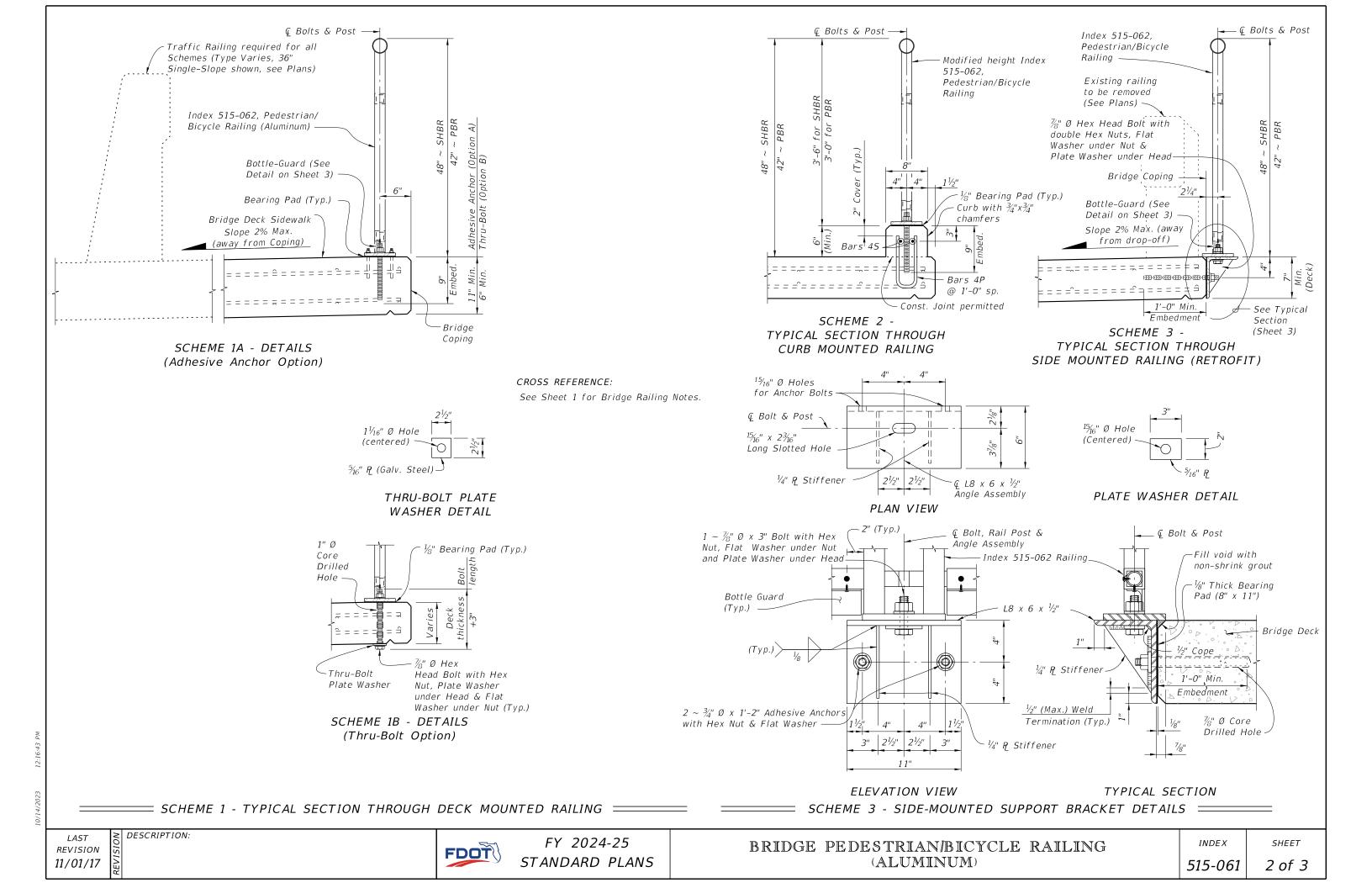
NOTES:

- Deck Joint at & Pier or Intermediate Bent similar.
- ** SHBR ~ Special Height Bicycle Railing PBR ~ Pedestrian/Bicycle Railing

- 1. Shop Drawings are required.
- 2. Work this Index with Index 515-062 Aluminum Bicycle/Pedestrian Railing Details and Specification Section 515. Refer to the IDS for Design Criteria and Limits of Use.
- 3. Materials:
 - A. Galvanized Steel Fasteners: Hex Head Bolt ASTM A307, Hex Nuts ASTM A563, Washers ASTM F436
 - B. Aluminum:
 - a. Support Bracket (Scheme 3) L-shape and Stiffener Plate: ASTM B209, Alloy 6061-T6
 - b. Bottle-guard (Schemes 1 & 3) L-shape: ASTM B209, Alloy 6061-T6 or 6063-T5
 - C. Concrete: Same as bridge deck
 - D. Pre-cured Silicone Sealant: Specification Section 932
 - E. Bearing Pads: Provide 1/8" thick Plain, Fabric Reinforced or Fabric Laminated pads meeting the requirements of Specification Section 932 for Ancillary Structures.
- 4. See Structures Plans, Superstructure Sheets for bridge information including concrete type, deck expansion joint locations and orientations, and thermal movement.
- 5. Railings:
 - A. For thermal movement greater than 4" (up to a maximum of 5"), clear opening between adjacent pickets, or panels at Rail Expansion Joints above Deck Joints must be reduced to $3\frac{1}{2}$ ".
 - B. For treatment of railings on skewed bridges see Index 521-427.
- 6. Curbs:
 - A. Match open curb joints at Deck Expansion Joint locations to the deck joint dimension.
 - B. Construct Concrete Curb (Scheme 2) vertical with the top surface finished level transversely. See Concrete Curb Details Sheet 3.
- C. Provide $\frac{3}{4}$ " Intermediate open joints in curbs coinciding with the $\frac{3}{4}$ "joints in the traffic railing.
- 7. Payment: Support bracket (Scheme 3) is incidental to the cost of railing. Curb concrete and reinforcing steel (Scheme 2) are included in the bridge deck quantities.

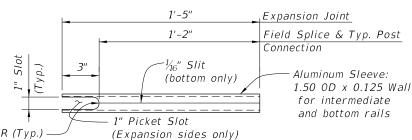
REVISION 11/01/17

DESCRIPTION:



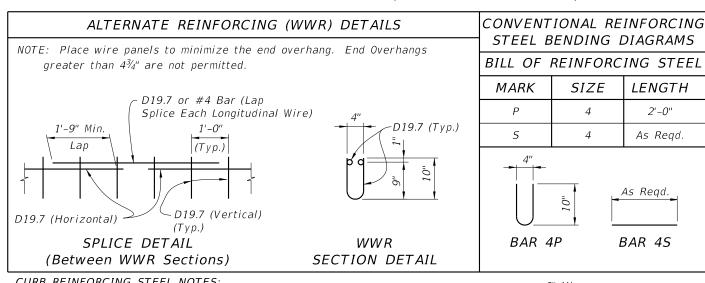
ROUND RAILS - TOP RAIL OR HANDRAIL

- * $\frac{1}{4}$ " Ø x $\frac{3}{4}$ " Pan Head Aluminum (Alloy 7075-T73) or Stainless Steel (Type 316 or 18-8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A 3/4" Ø plug weld may be substituted for the two set screws at expansion joints.
- ** Embedded length may be 4" for plug welded connection.
- *** Increase handrail sleeve embedment to 8" for Expansion Joint openings greater than 2".
- **** Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".



INTERMEDIATE OR BOTTOM RAIL - ALUMINUM SLEEVE DETAIL (Bottom Side Shown)

DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =



CURB REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Deformed WWR meeting the requirements of Specifications Section 931 may be used in lieu of all Bars 4P and 4S.

Pre-cured Silicone Sealant (4" wide) 6" Min.	
over.	
1 931	

DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

QUANTITIES (SCHEME 2)

ESTIMATED CONCRETE CURB

ITEM	UNIT	QUANTIT
Concrete	CY/LF	0.0124
Reinforcing Steel	LB/LF	4.01

INTERMEDIATE JOINT SEAL NOTE:

At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

SCHEME 2 - CONCRETE CURB DETAILS =



FY 2024-25 STANDARD PLANS



INDEX

î Rail, Bottle-

Guard and Screws

#10 x 5/4" Pan Head

Screws (18-8 SS)

@ 2'-0" sp.

TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME1 - BOTTLE GUARD DETAIL =

Bottom Rail

Bottle Guard

TYPICAL SECTION THROUGH BOTTOM RAIL

(Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL =

1½" x 2½" x ½" L

Bottom Rail

Bottle Guard

1½" x 1½" x ⅓" ∠

SHEET

REVISION 11/01/16 DESCRIPTION:

BRIDGE PEDESTRIAN/BICYCLE RAILING

© Rail, Bottle-Guard and Screws

#10 x 5/4" Pan Head

Screws (18-8 SS)

@ 2'-0" sp. —

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

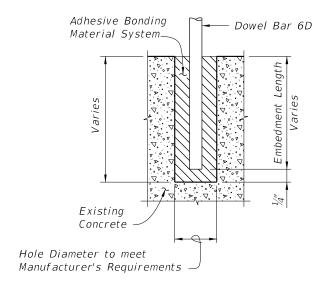
BRIDGES ON CURVED ALIGNMENTS: The details presented in this Standard are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing along the entire length of the bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

GUARDRAIL: See Index 536-001 for guardrail component details, geometric layouts and associated notes not fully detailed

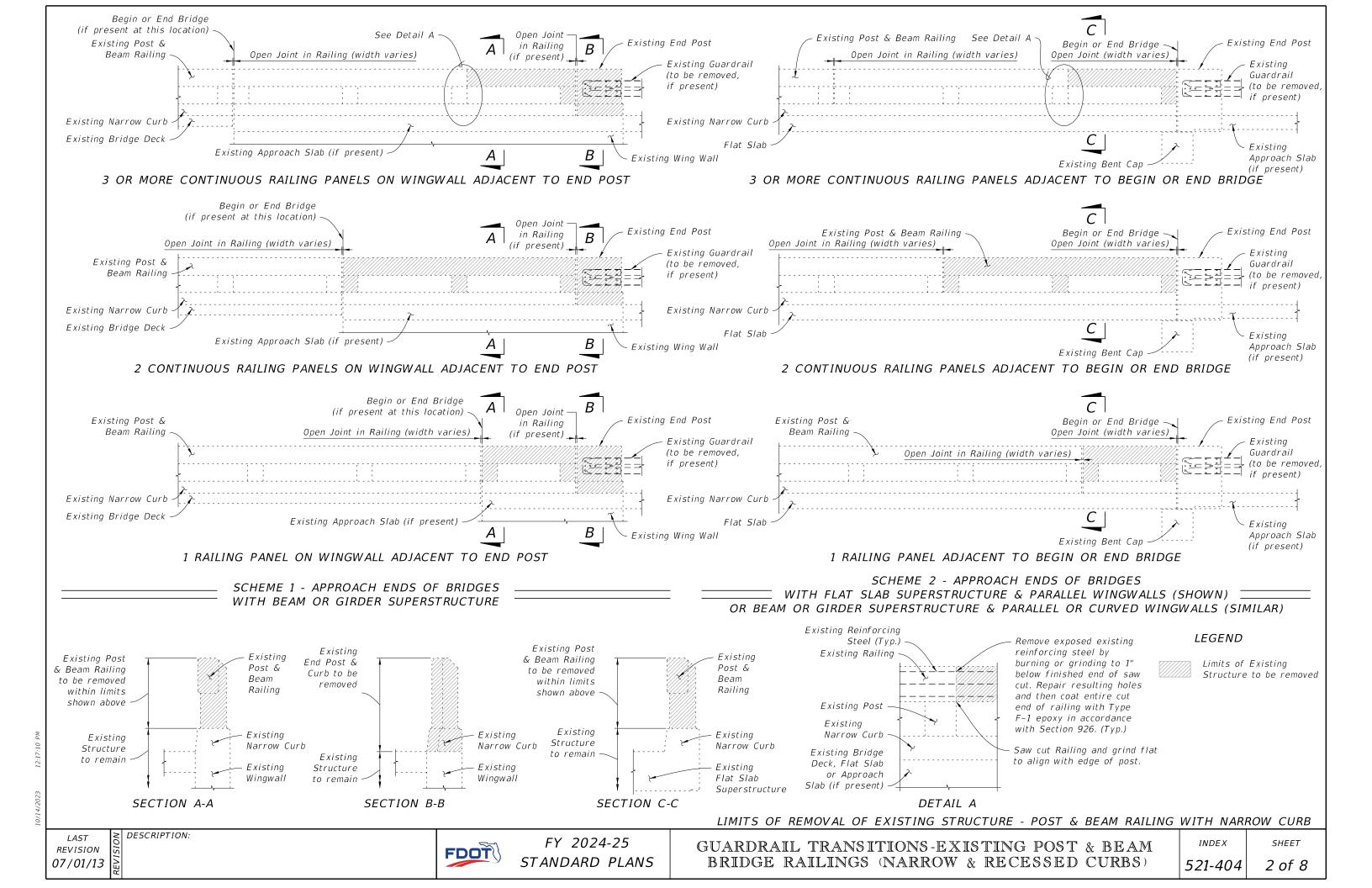
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent quardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise individual decals of letters and numbers.

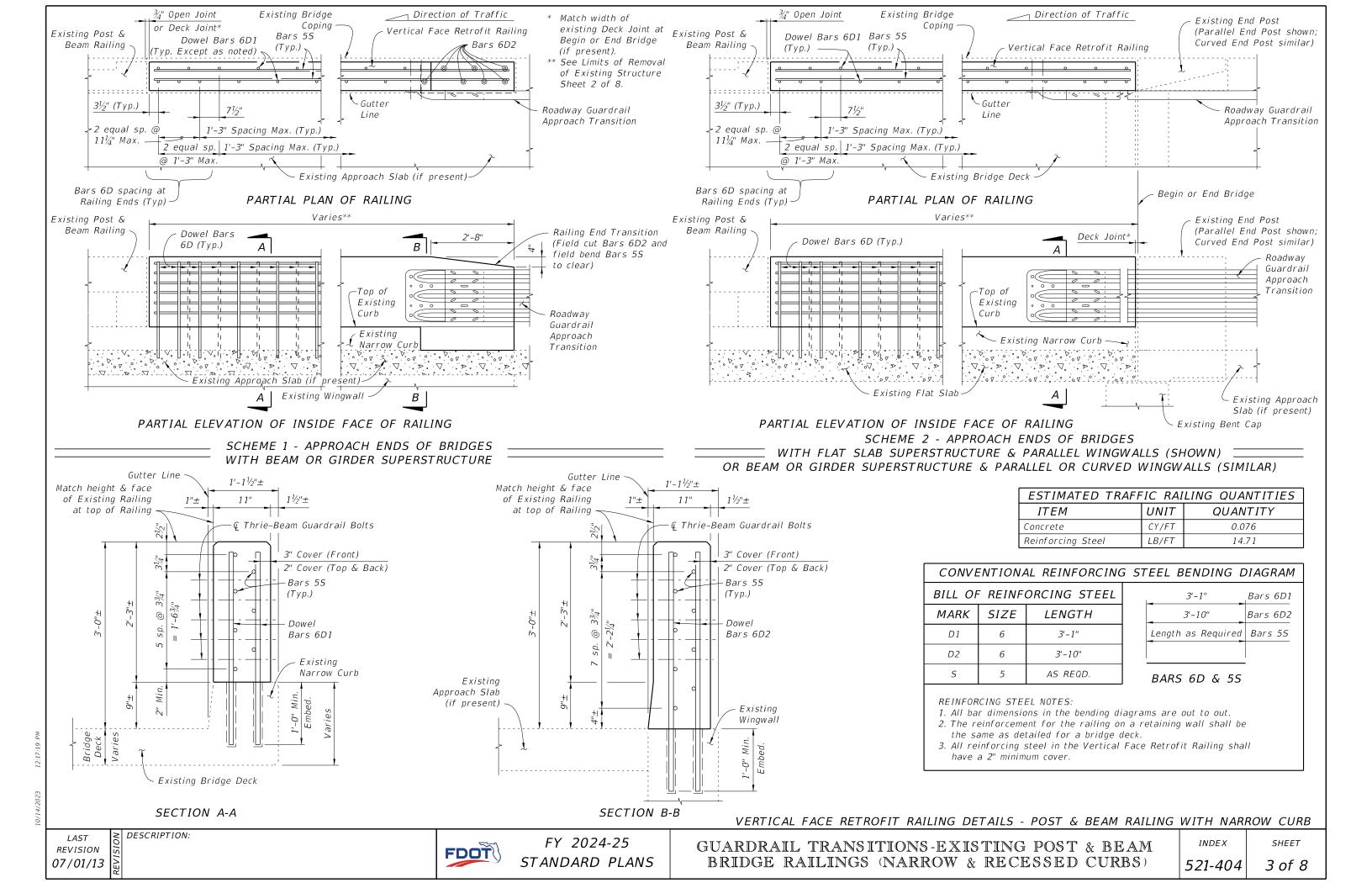
PAYMENT: Concrete Traffic Railing-Bridge Retrofit - Post & Beam Railing (EA) includes all material and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railing, Guardrail Approach Transition to rigid Barriers (EA) includes transition block, and necessary hardware to complete the Guardrail transitions shown.

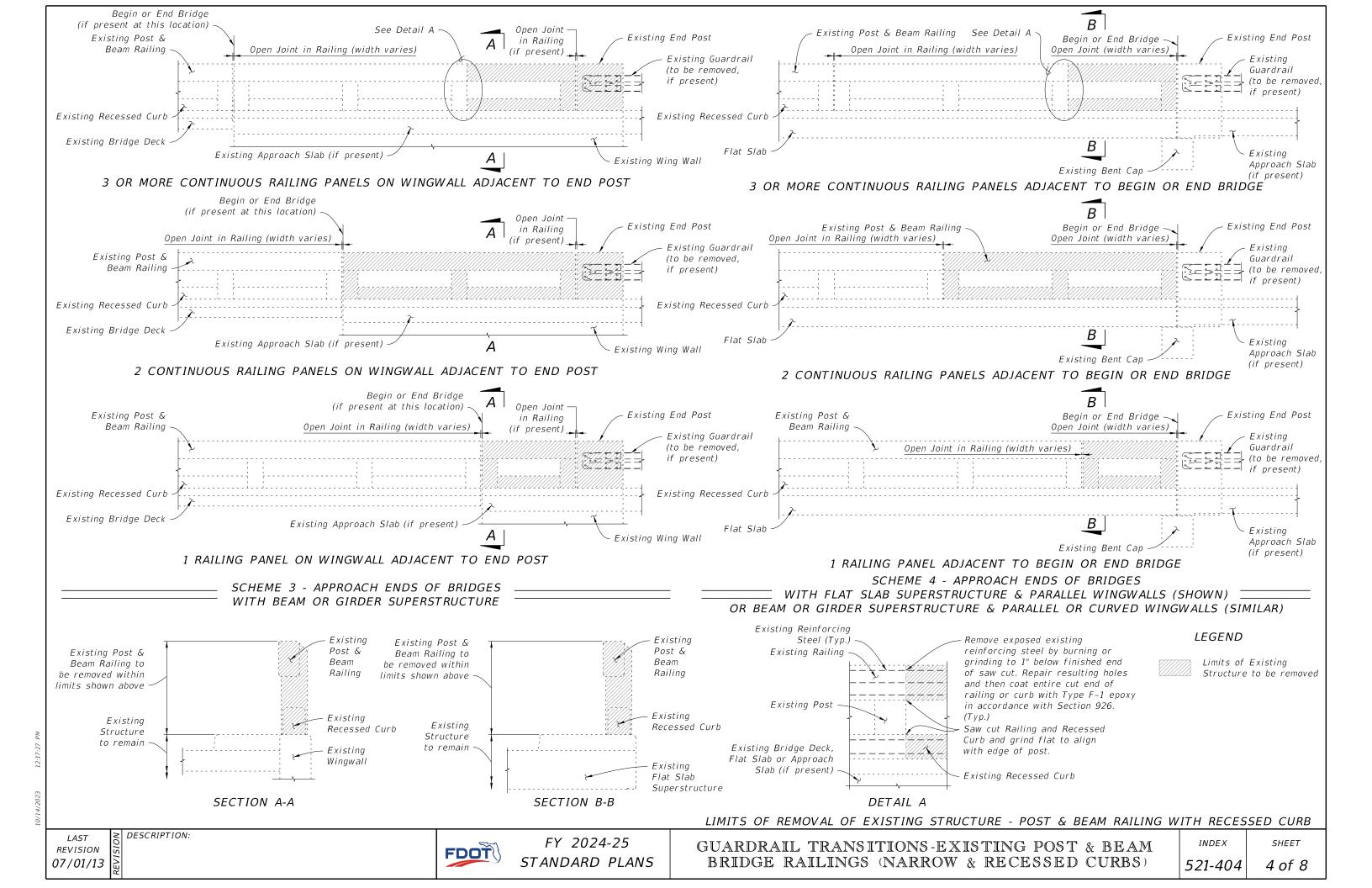


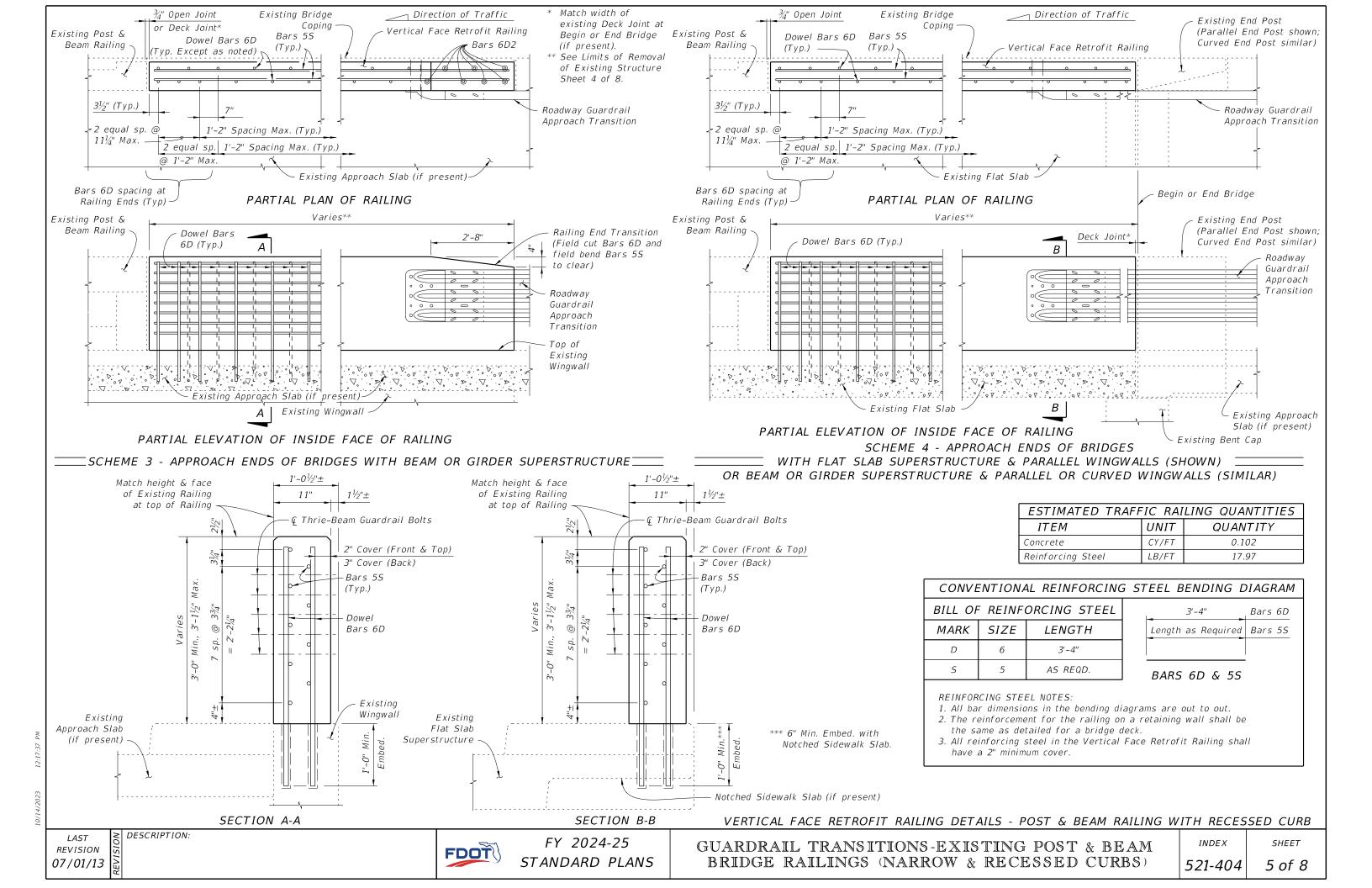
DOWEL DETAIL

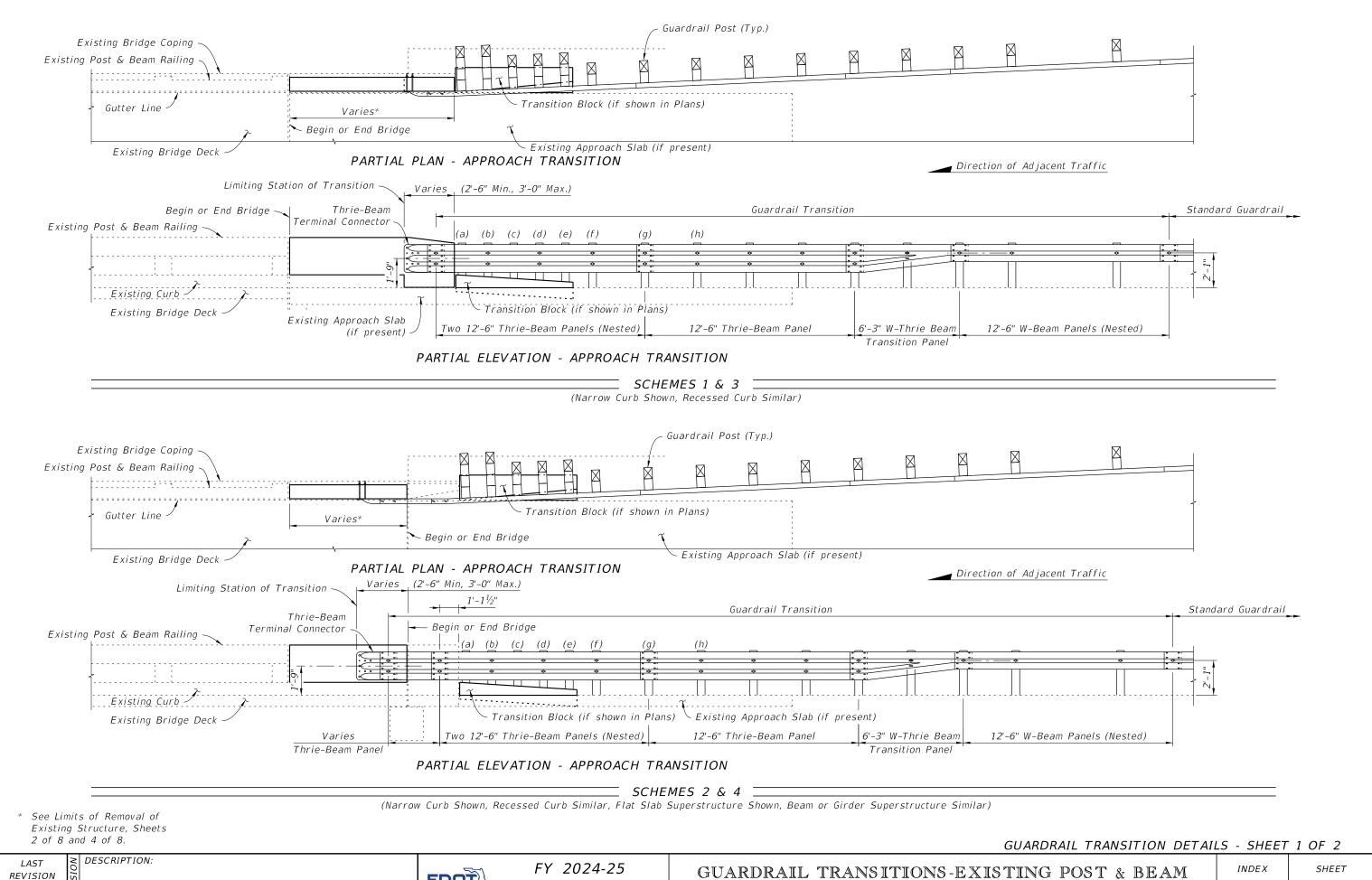
Note: Shift dowel holes to clear if the existing reinforcement is encountered.











07/01/14

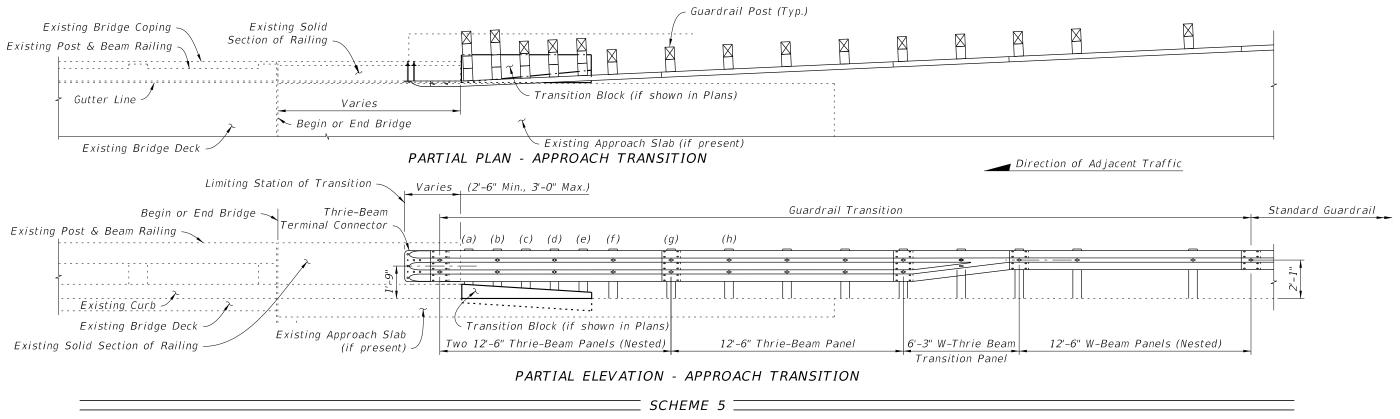
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STANDARD PLANS

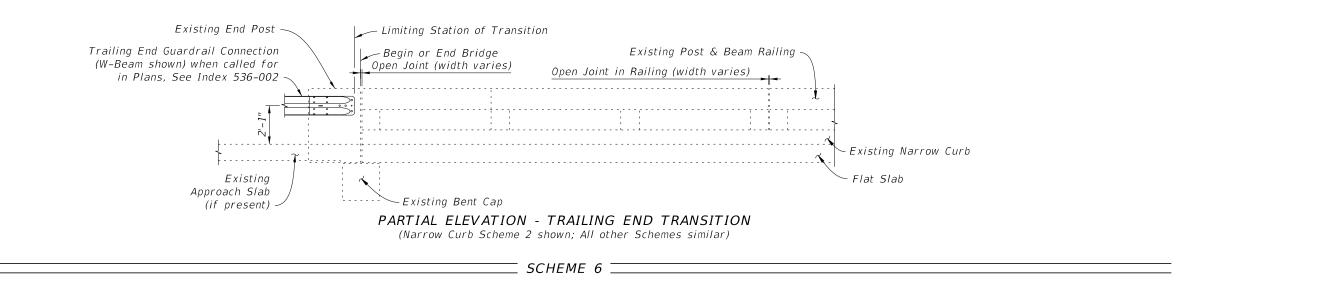
BRIDGE RAILINGS (NARROW & RECESSED CURBS)

521-404

6 of 8



(Narrow Curb shown; Recessed Curb similar)



DESCRIPTION: REVISION 07/01/14

FDOT

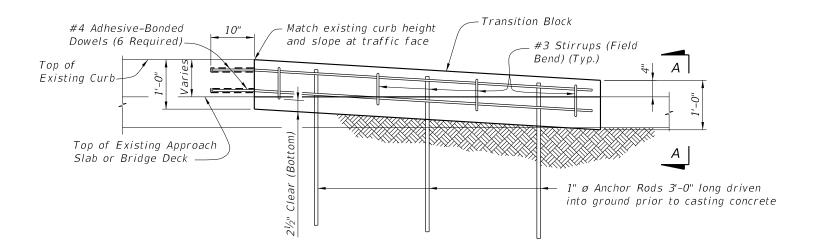
FY 2024-25 STANDARD PLANS

GUARDRAIL TRANSITIONS-EXISTING POST & BEAM

INDEX *521-404*

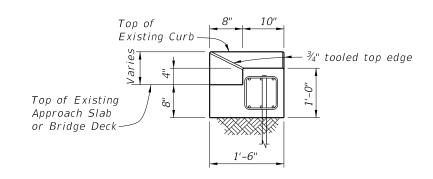
SHEET 7 of 8

PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

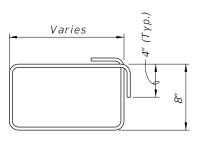


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK			
ITEM UNIT QUANTITY			
Concrete Class II (Bridge Deck)	CY	0.4	
Reinforcing Steel	LB	61	



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

ADHESIVE-BONDED DOWELS: Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

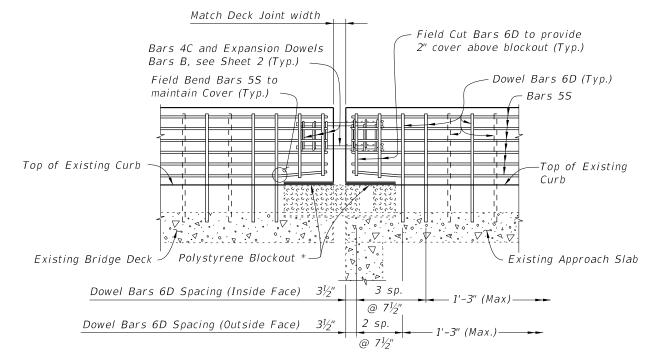
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install barrier delineators on top of the Traffic Railing along the entire length of bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Concrete Traffic Railing - Bridge Retrofit - Post & Beam Railing (each) includes all materials and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railings. Guardrail Approach Transition to Rigid Barriers (EA) includes all transition blocks, and necessary hardware to complete the Guardrail transitions shown.

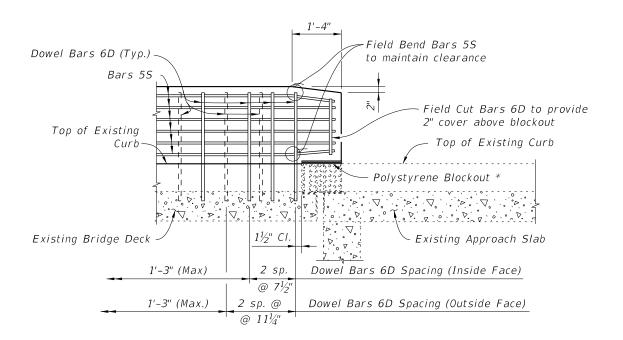
ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUAN	NTITY
I I E M	ONT	9" Curb	Increment
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.)



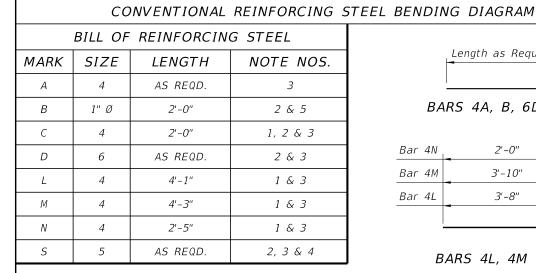
PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEMES 2 THRU 5

* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



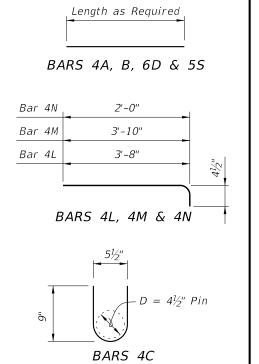
PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE
JOINT AT BEGIN OR END BRIDGE - SCHEME 1
(Guardrail Transition not shown for clarity)

12:18:12 P

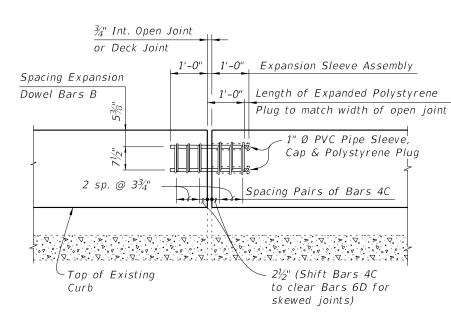


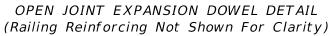
REINFORCING STEEL NOTES:

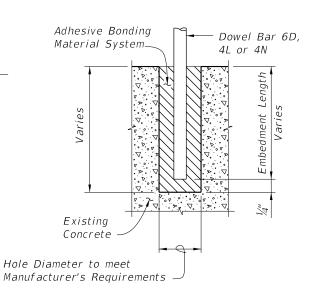
- 1. All bar dimensions in the bending diagrams are out to out. 2. The reinforcement for the railing on a retaining wall shall
- be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.



(12 required per open joint)



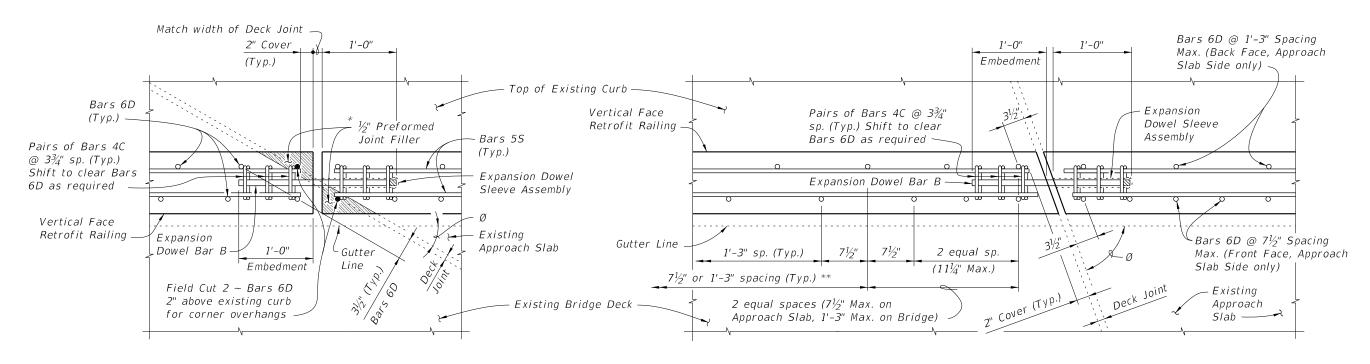




DOWEL DETAIL

Dowel Installation Note: Shift dowel holes to clear if the existing reinforcement is encountered.

* $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.



PARTIAL PLAN OF RAILING (SKEW ANGLE Ø LESS THAN 70°)

PARTIAL PLAN OF RAILING (SKEW ANGLE $\emptyset = 70^{\circ}$ OR GREATER)

SKEW DETAIL

REVISION 07/01/13

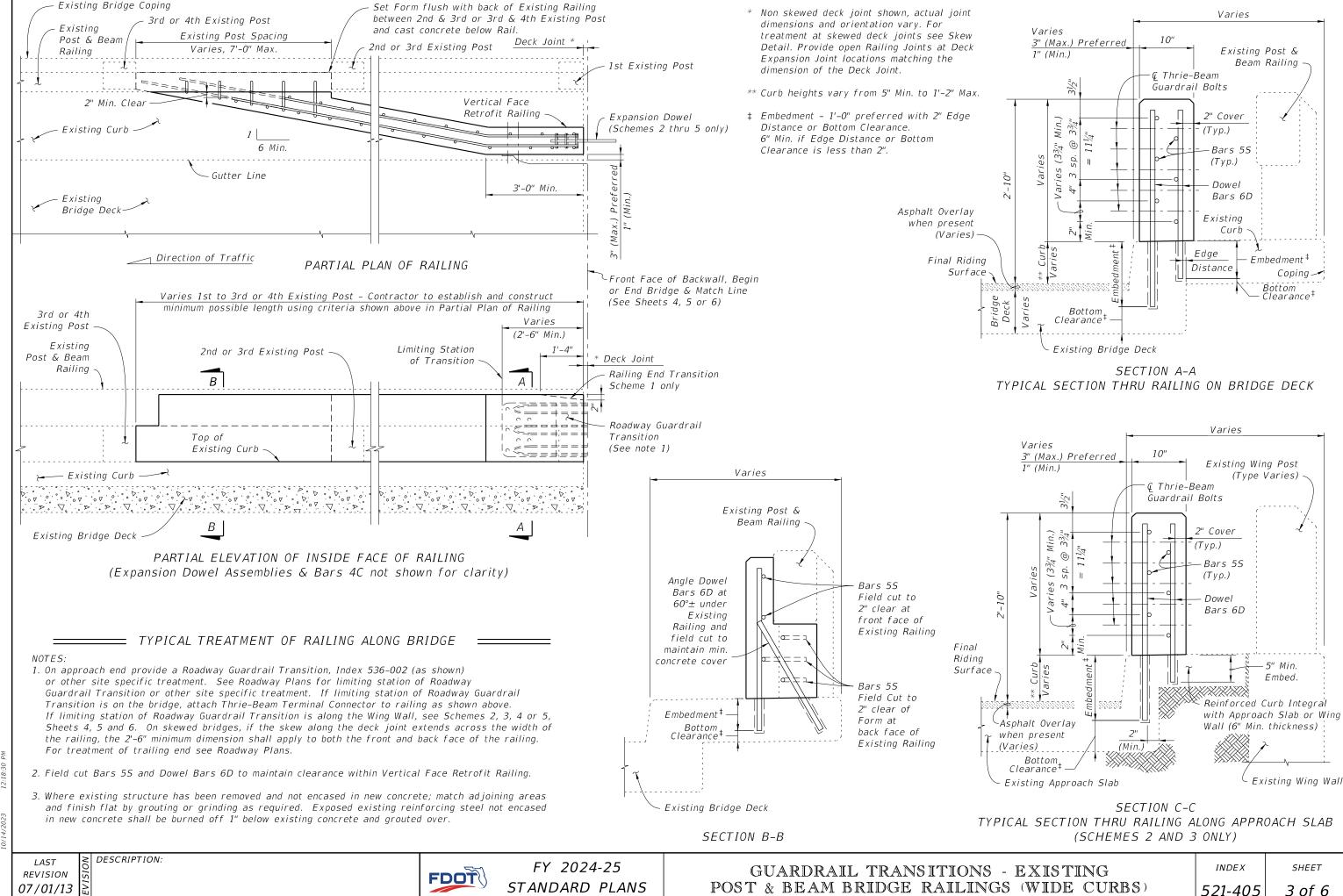
DESCRIPTION:

FY 2024-25 STANDARD PLANS

GUARDRAIL TRANSITIONS - EXISTING POST & BEAM BRIDGE RAILINGS (WIDE CURBS) INDEX

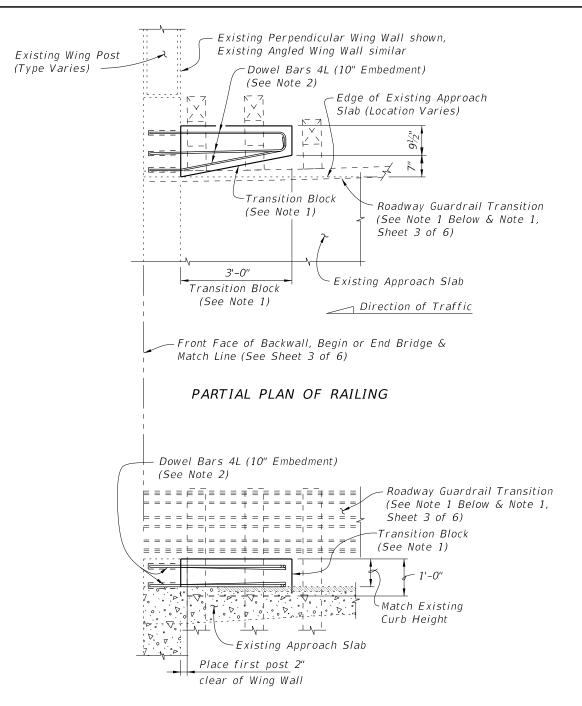
SHEET

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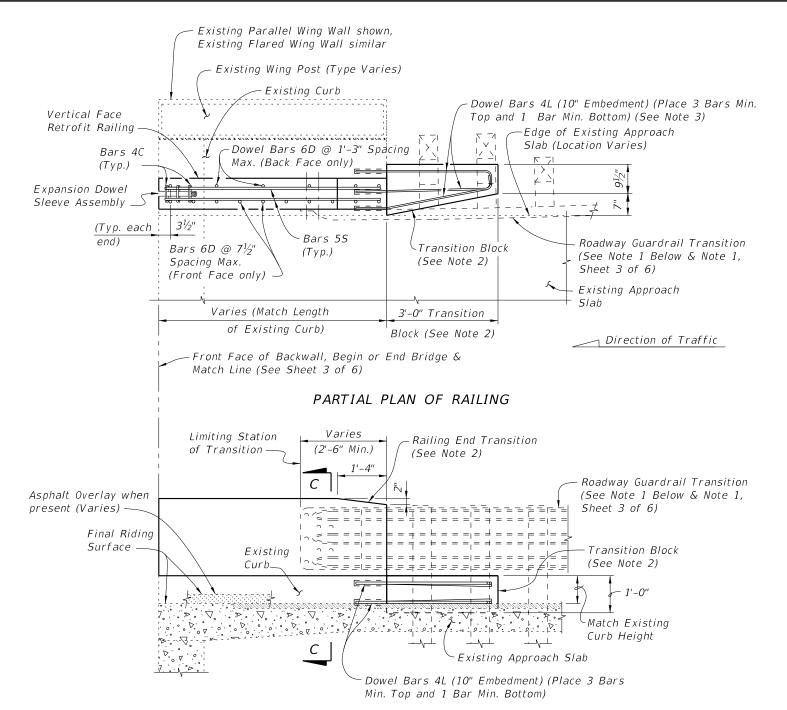


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

=== SCHEME 1 === RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> ______ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 3 of 6. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

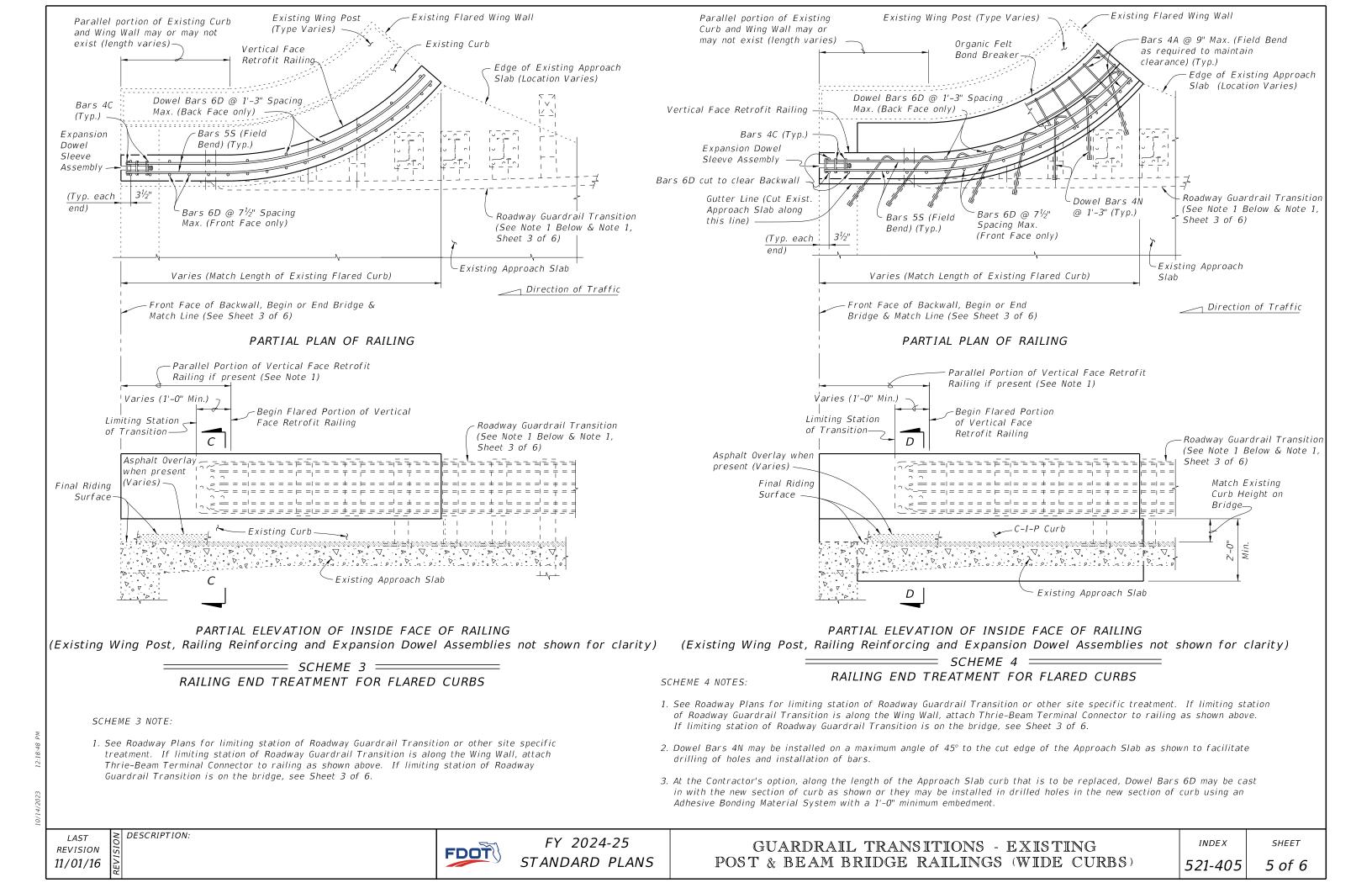
DESCRIPTION: REVISION 07/01/13



FY 2024-25 STANDARD PLANS

GUARDRAIL TRANSITIONS - EXISTING POST & BEAM BRIDGE RAILINGS (WIDE CURBS) *INDEX*

SHEET



SCHEME 5 NOTES:

DESCRIPTION:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 3 of 6.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.

FY 2024-25 STANDARD PLANS



INDEX *521-405*

Varies

2" Clear

Bars 5S (Typ.)

Dowel Bars 6D

(See Note 5)

(Typ.)

Existing Wing Post

(Type Varies)

Organic Felt

Bars 4A @ 9" Max.

2" Min. Clear. Top

and Sides, 4" Min.

Clear. Bottom +

Bars 4A @ 9" Max., Min. 3 full length bars

required Top & Bottom (Field Bend to clear) (Typ.)

Bond Breaker

Varies (1'-2" Min.)

⊊ Thrie-Beam

Guardrail Bolts

Varies

1" Min.

Final Riding

Dowel Bars 4N @ 1'-3" (Typ.)

Gutter Line (Cut

Existing Approach

Portion of Existing Approach

Slab with Integral Curb less than 6" thick or portion of

Existing Approach Slab and

Curb with Floating Detached Sidewalk to be removed

shown hatched.

Existing Approach Slab

Asphalt Overlay

when present (Varies)

Slab along this line) _

Surface

Asphalt Overlay

when present

Existing Approach Slab

(Varies)

3" (Max.) Preferred

(3¾"

Embedmen

SECTION D-D

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

Existing Wing Post

Varies (1'-2" Min.)

(Type Varies)

Wing Wall

TYPICAL SECTION THRU EXISTING APPROACH

SLAB AND END BENT WING WALL SHOWING LIMITS OF REMOVAL

(SCHEMES 4 AND 5 ONLY)

Varies

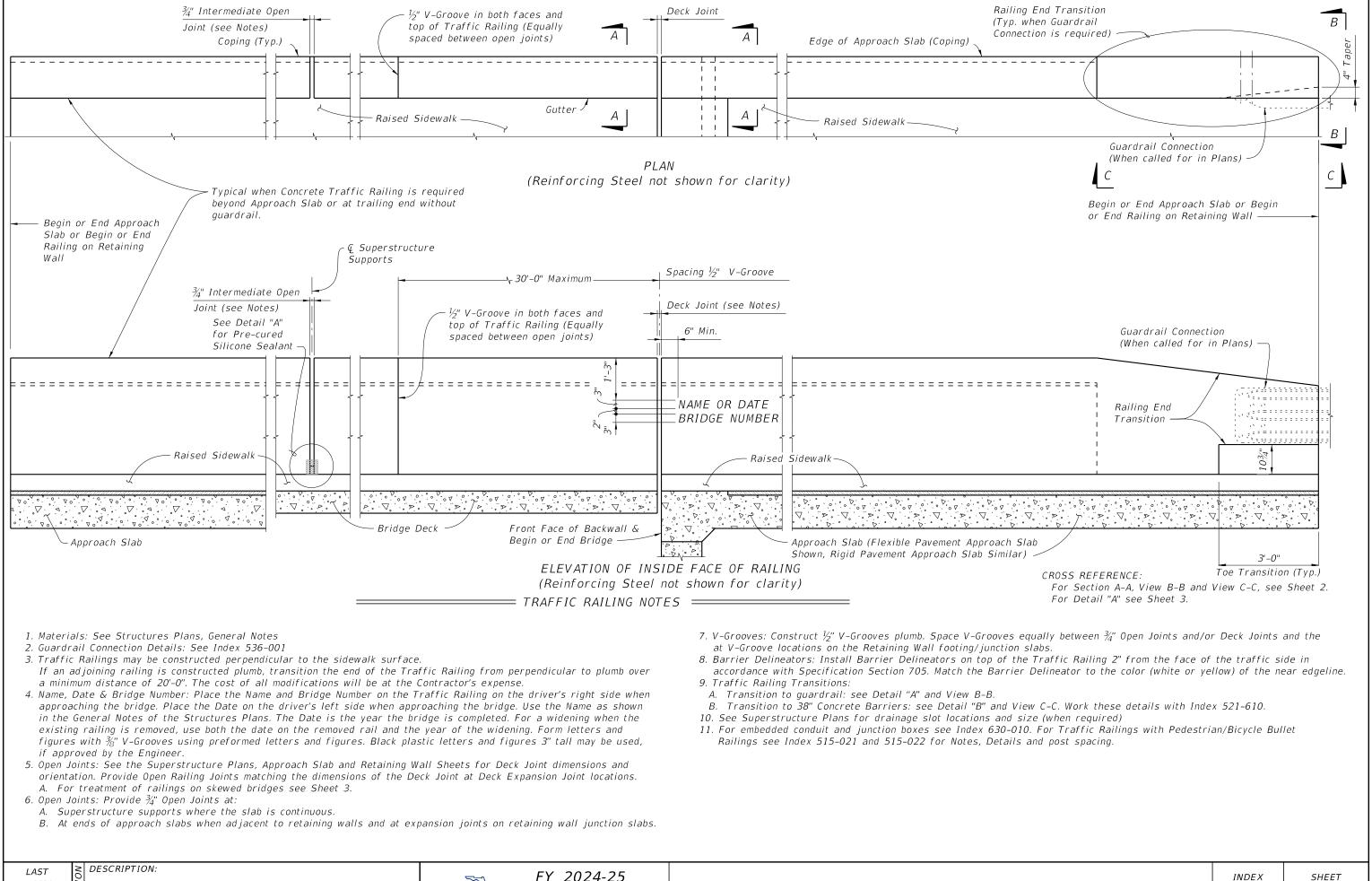
SHEET 6 of 6

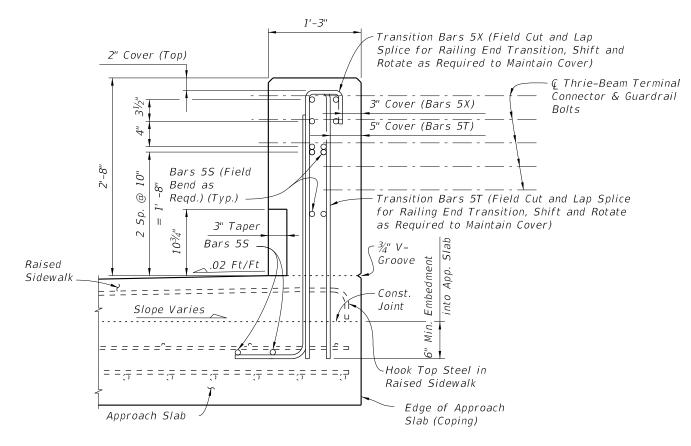


REVISION

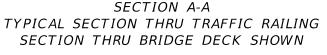
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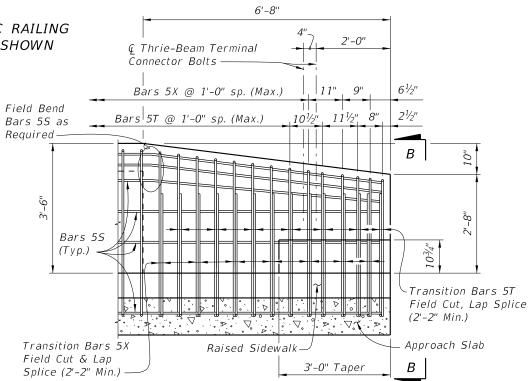


VIEW B-B (END VIEW OF TRAFFIC RAILING END TRANSITION) (Approach Slab shown, Retaining Wall Junction Slab similar)



1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover

2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)



VIEW C-C RAILING END TRANSITION (Guardrail Not Shown For Clarity) CROSS REFERENCE: For location of Section A-A, View B-B and View C-C, see Sheet 1.

DESCRIPTION:

REVISION

11/01/17

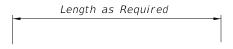
in Railing End Transition.

NOTES:

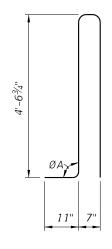
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL			
MARK SIZE LENGTH			
S	5	As Reqd.	
Т	5	10'-8"	
Х	5	6'-9"	

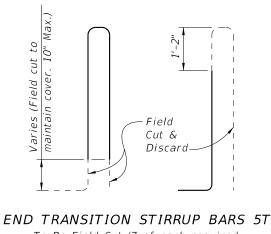
ROADWAY	ØA		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	83°	
6% to 10%	84°	96°	



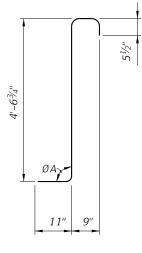
BAR 5S

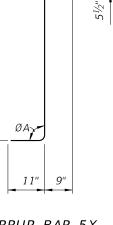


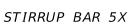
STIRRUP BAR 5T

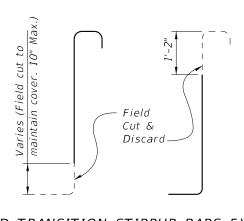








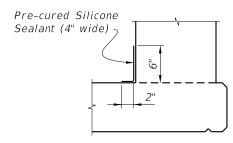




END TRANSITION STIRRUP BARS 5X To Be Field Cut (7 of each required per Railing End Transition)

REINFORCING STEEL NOTES:

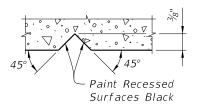
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The $4'-6^{3}/4''$ vertical dimension shown for Bars 5T and 5X is based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slope vary from the above amounts, adjust this dimension accordingly to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above with
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.

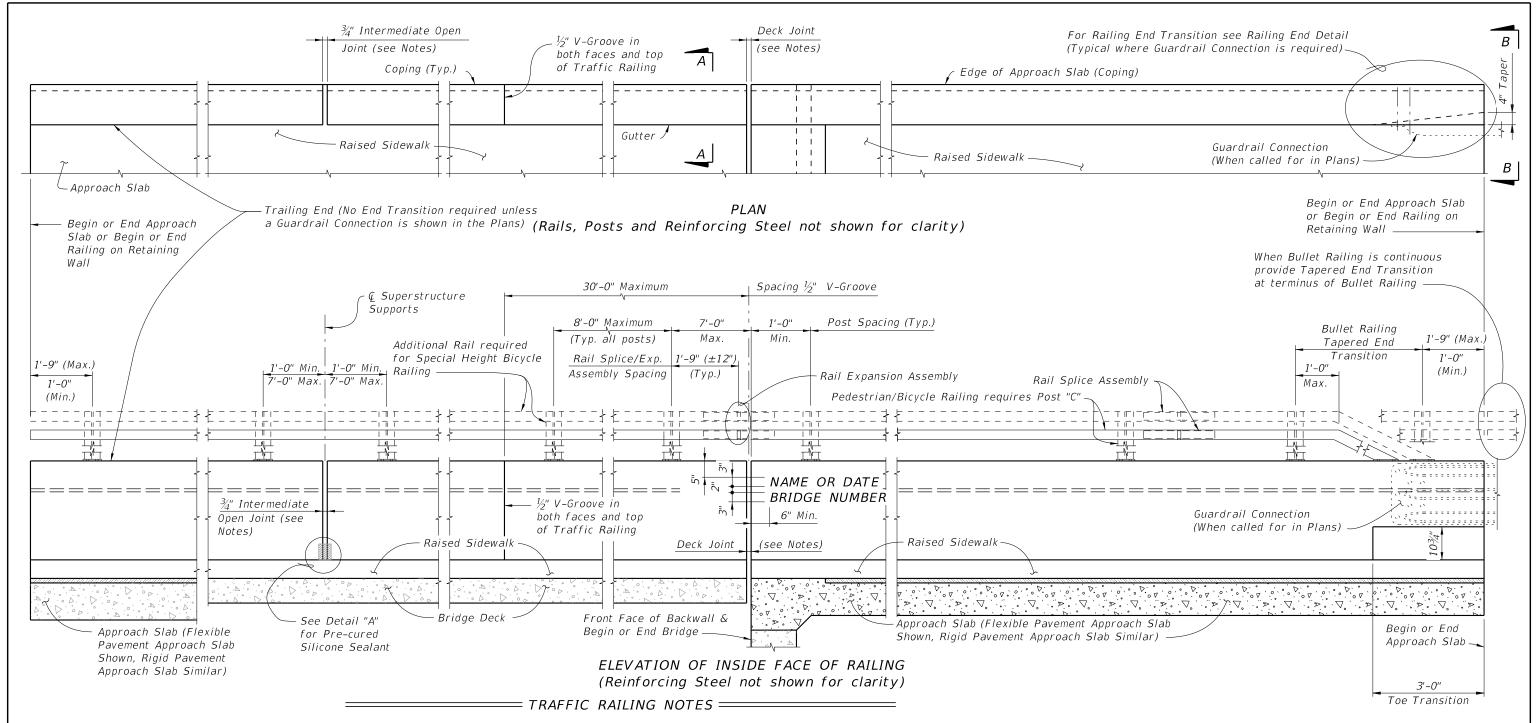


SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM UNIT QUANTITY				
Concrete CY/LF 0.145				
Reinforcing Steel	LB/LF	30.68		

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)





- 1. Materials: See Structures Plans, General Notes
- 2. Guardrail Connection Details: See Index 536-001
- 3. Traffic Railings may be constructed perpendicular to the sidewalk surface.

 If an adjoining railing is constructed plumb, transition the end of the Traffic Railing from perpendicular to plumb over a minimum distance of 20'-0". The cost of all modifications will be at the Contractor's expense.
- 4. Name, Date & Bridge Number: Place the Name and Bridge Number on the Traffic Railing on the driver's right side when approaching the bridge. Place the Date on the driver's left side when approaching the bridge. Use the Name as shown in the General Notes of the Structures Plans. The Date is the year the bridge is completed. For a widening when the existing railing is removed, use both the date on the removed rail and the year of the widening. Form letters and figures with 3/8" V-Grooves using preformed letters and figures. Black plastic letters and figures 3" tall may be used, if approved by the Engineer.
- 5. Open Joints: See the Superstructure Plans, Approach Slab and Retaining Wall Sheets for Deck Joint dimensions and orientation. Provide Open Railing Joints matching the dimensions of the Deck Joint at Deck Expansion Joint locations. A. For treatment of railings on skewed bridges see Sheet 3.
- 6. Open Joints: Provide 3/4" Open Joints at:

DESCRIPTION:

- A. Superstructure supports where the slab is continuous.
- B. At ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.

- 7. V-Grooves: Construct $\frac{1}{2}$ " V-Grooves plumb. Space V-Grooves equally between $\frac{3}{4}$ " Open Joints and/or Deck Joints and the at V-Groove locations on the Retaining Wall footing/junction slabs.
- 8. Barrier Delineators: Install Barrier Delineators on top of the Traffic Railing 2" from the face of the traffic side in accordance with Specification Section 705. Match the Barrier Delineator to the color (white or yellow) of the near edgeline.
- 9. For embedded conduit and junction boxes see Index 630-010.
- 10. For Traffic Railings with Pedestrian/Bicycle Bullet Railings see Index 515-021 and 515-022 for Notes, Details and post spacing.

CROSS REFERENCE:
For Section A-A and
View B-B, see Sheet 2.
For Detail "A" see Sheet 3

LAST REVISION 11/01/20

FDOT

FY 2024-25 STANDARD PLANS

TRAFFIC RAILING - (32" VERTICAL SHAPE)

INDEX **521-423**

SHEET

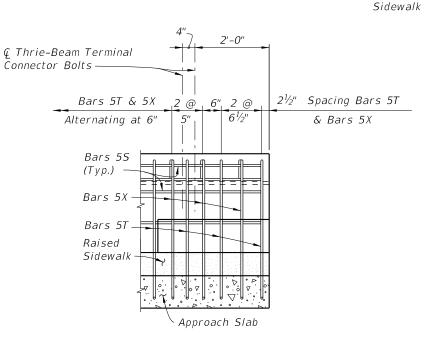
1 of 2

SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING (Section Thru Bridge Deck shown)

NOTES:

DESCRIPTION:

- 1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.
- 2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)



Approach 1'-0" Slab VIEW B-B APPROACH SLAB END VIEW OF TRAFFIC RAILING

Bars 5S

1'-1"

-Bars 5X @ 1'-0" sp. (Max.) (Alternate with

Bars 5T) (See Note 1)

← Thrie-Beam Terminal

Connector & Guardrail

Bars 5S (Field Bend as

Bars 5T @ 1'-0" sp. (Max.)

(Alternate with Bars 5X)

Required) (Typ.)

(See Note 1)

CROSS REFERENCE: For location of Section A-A and View B-B see Sheet 1.

Const

Joint

Hook Top Steel in

Edge of Approach

Raised Sidewalk

Slab (Coping)

NOTE: For Bullet Railing Details, see Index 515-022.

RAILING END DETAIL (Guardrail Not Shown For Clarity)

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SHEET 2 of 3

Additional Rail required for

Bicycle

(Pedestrian/Bicycle

Raised

Special Height Bicycle Railing

Pedestrian/Bicycle Railing

2" Cover (Top)

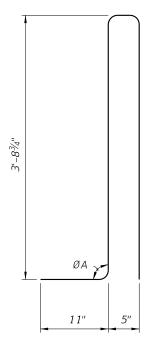
3" Taper

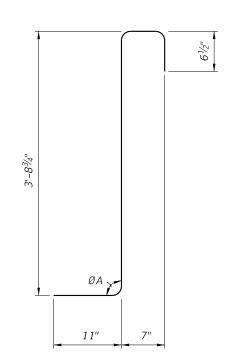
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CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK SIZE LENGTH				
5	5	As Reqd.		
Т	5	9'-0"		
Χ	5	5'-10"		

ROADWAY	ØA		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	9 <i>3</i> °	
6% to 10%	84°	96°	





Length as Required

BAR 5S

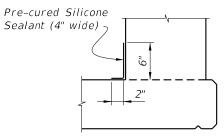
STIRRUP BAR 5T

DESCRIPTION:

STIRRUP BAR 5X

REINFORCING STEEL NOTES:

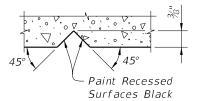
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 3'-8¾" vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into the bridge deck.
- 3. The reinforcement for the railing on a Retaining Wall shall be the same as detailed with $\emptyset A = 90^{\circ}$.
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.

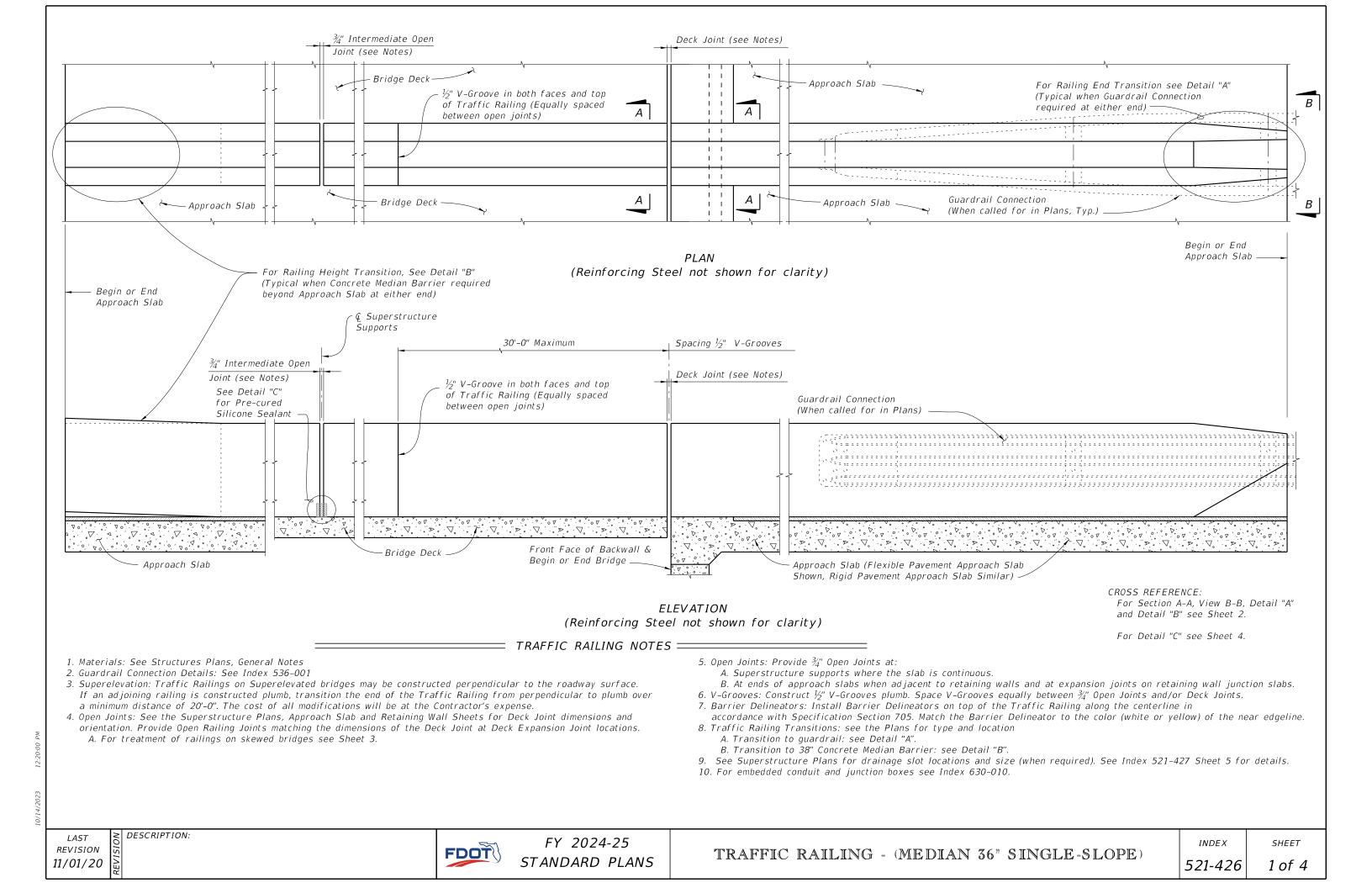


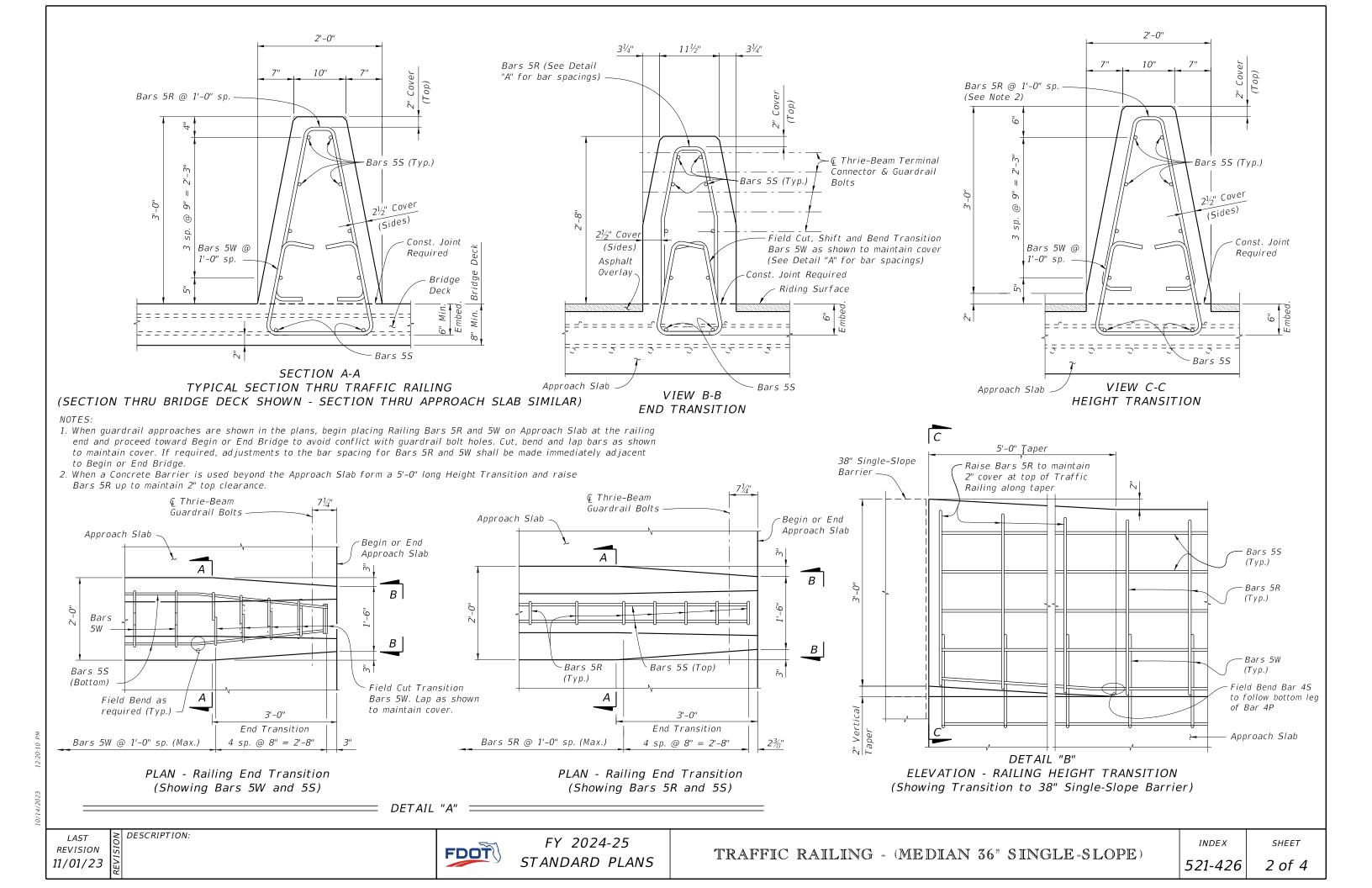
SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.095	
Reinforcing Steel	LB/LF	25.90	

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope.)





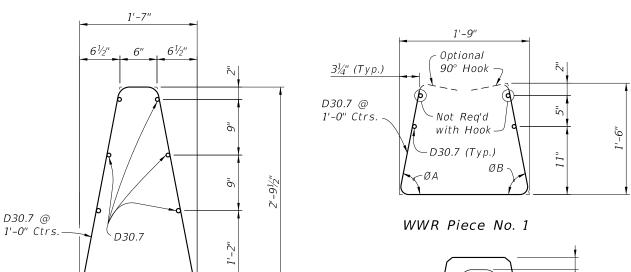


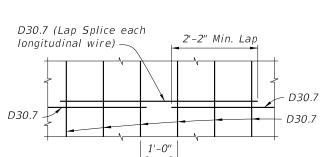
PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4) $\frac{3}{4}$ " Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at & Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

ALTERNATE REINFORCING STEEL (WWR) DETAILS





(Between WWR Sections)

WELDED WIRE REINFORCEMENT NOTES:

of Piece 1 shall be cut to allow overlap.

DESCRIPTION:

Overhangs greater than 6" are not permitted.

21/2" Cover 6" (Sides) WWR Piece No. 2 WWR Piece No. 2 - WWR Piece No. 1 SPLICE DETAIL SECTION A-A

1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 5R,

2. WWR at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The bottom

3. Place WWR panels so as to minimize the end overhang of longitudinal wires at Railing Ends and Open Joints.

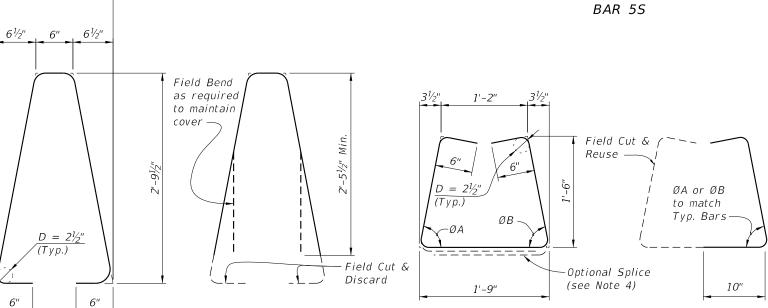
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

ROADWAY	ON SLOPE		AT CROWN		
	CROSS-SLOPE	ØA	ØB	ØA	ØB
	0% to 2%	79°	79°	79°	79°
	>2% to 6%	81°	77°	79°	79°
	>6% to 10%	84°	74°	79°	79°

ØA and ØB shall be 79° if Contractor elects to place railing perpendicular to the deck, and approach slabs.

BILL OF REINFORCING STEEL			
MARK SIZE LENGTH			
R	5	7'-2"	
S	5	As Reqd.	
W	5	5'-10"	

Length as Required



STIRRUP BAR 5R TRANSITION STIRRUP BAR 5R (5 required per Railing End Transition)

STIRRUP BAR 5W

TRANSITION STIRRUP BAR 5W To Be Field Cut (10 required per Railing End Transition)

REINFORCING STEEL NOTES:

1'-7"

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 4. At the Contractor's option, Bars 5W may be fabricated as a two piece bar with a 1'-2" lap splice of the bottom legs.

Pre-cured Silicone Sealant 4" wide (Typ.) _(Typ.)

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

5S and 5W. WWR must meet the requirements of Specification Section 931.

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM UNIT QUANTITY			
Concrete	CY/LF	0.157	
Reinforcing Steel	LB/LF	23.99	

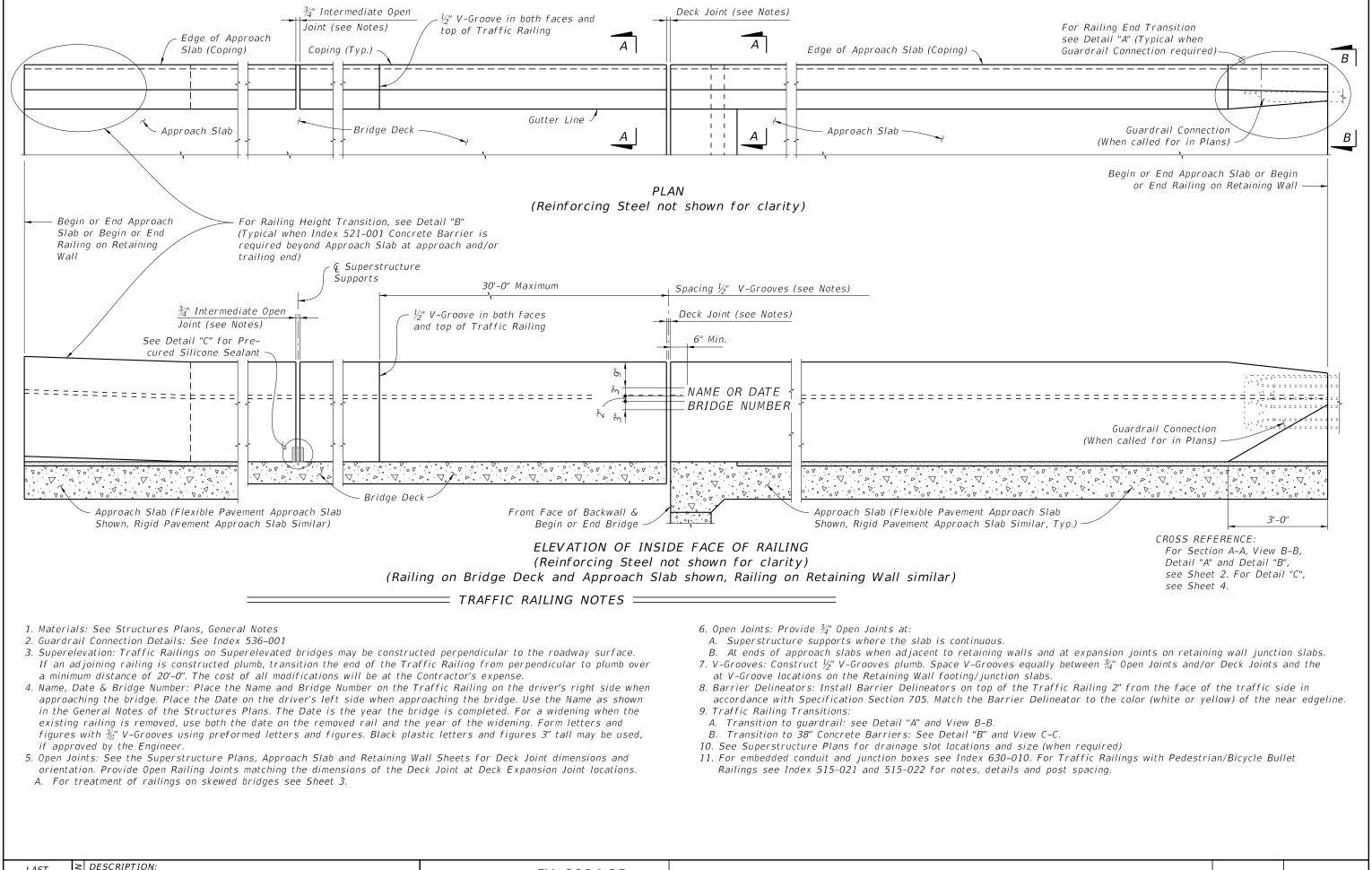
(The above quantities are based on a crowned roadway, with a 2% cross slope)

REVISION 01/01/18

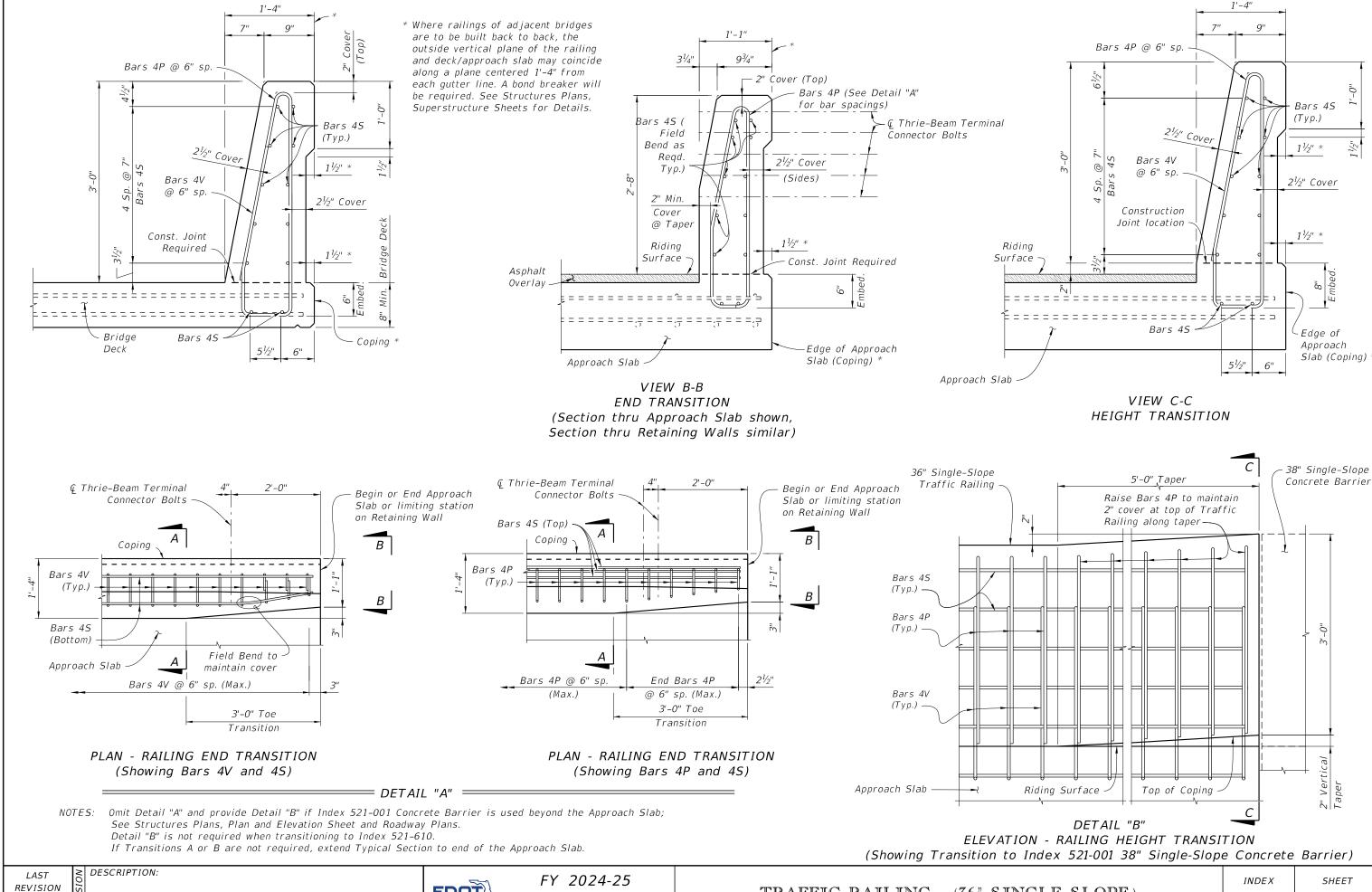


FY 2024-25 STANDARD PLANS

SHEET 4 of 4

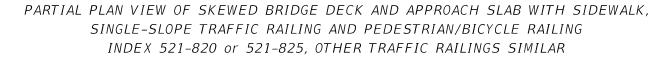


SHEET



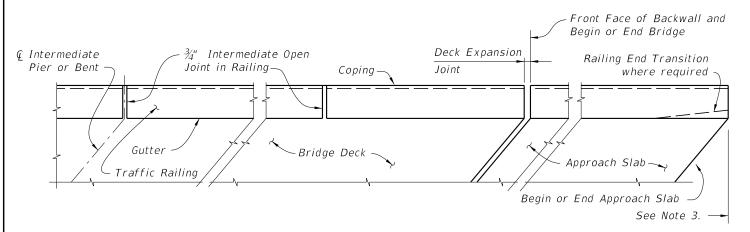
11/01/20

FDOT



NOTES:

- 1) Concrete Parapet reinforcement is not effected by skew angle, see Index 521-820 for details.
- 2) Parapet expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 3) Traffic Railing reinforcement vertical Bars 4V & 4P may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement. Bars 4V adjacent to expansion joints shall be field adjusted to maintain clearance and spacing, extra Bars 4V will be required. Cut bottom horizontal portion of 4V Bars to maintain maximum horizontal length to each vertical leg being placed. Discard the remainder of the bar. Rotate cut bars to maintain clearance.
- 4) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 5) $\frac{3}{4}$ " Intermediate Open Joints and V-Grooves in railing and parapet shall be placed perpendicular or radial to the gutter line or inside face of parapet line. See Structures Plans, Superstructure Sheets for locations.
- 6) At begin or end approach slab extend slab at the railing ends 3" (gutter side or back face of railing as required) as shown to provide a base for casting of the railing. Field trim toe of Bars 4V by 1 inch as required to maintain concrete cover at edge of deck.
- 7) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.



PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SINGLE-SLOPE TRAFFIC RAILING, OTHER TRAFFIC RAILINGS SIMILAR

NOTES:

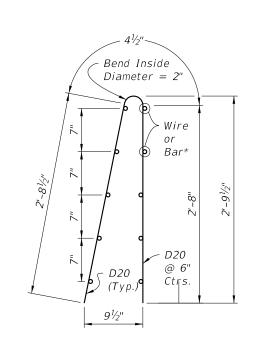
- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 2) $\frac{3}{4}$ " Intermediate Open Joints and $\frac{1}{2}$ " V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 3) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.

GENERAL NOTES:

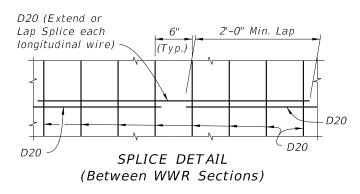
- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes as applicable
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing.
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

ALTERNATE REINFORCING STEEL (WWR) DETAILS

*Longitudinal D20 Wires or #4 Bars may be tied.



WWR Piece No. 2



WELDED WIRE REINFORCEMENT NOTES:

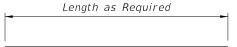
- 1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 4P, 4S and 4V. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.
- 2. WWR at Railing End Transition shall be field bent inward as required (Piece 2) to maintain cover. The bottom of the vertical wires (D20) in Piece 2 shall be cut a maximum of 4 inches and the gutter side portion bent inward as required to allow placement.

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

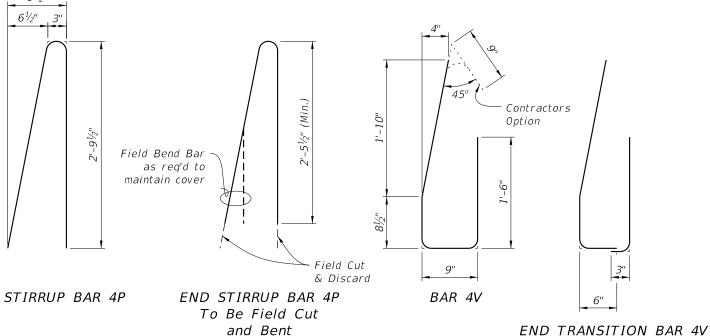
ROADWAY	LOW GUTTER	HIGH GUTTER
CROSS-SLOPE	ØB	ØB
0% to 2%	90°	90°
2% to 6%	87°	9 <i>3</i> °
6% to 10%	84°	96°

BILL OF REINFORCING STEEL		
MARK	SIZE	LENGTH
Р	4	5'-11"
S	4	As Reqd.
V	4	4'-10"

ØB shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.







REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The $8\frac{1}{2}$ " vertical dimensions shown for Bar 4V is based on a 6" embedment into the bridge deck without a raised sidewalk. If a raised sidewalk is to be provided, increase this dimension to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- All reinforcing steel at the open joints shall have a 2" minimum cover.
- Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 2'-0".

Pre-cured Silicone Sealant (4" wide)

DESCRIPTION:

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

WWR Piece No. 2

21/2" Cover

1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.

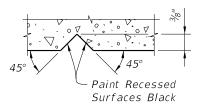
9"

WWR Piece No. 1

2½" Cover

WWR Piece No. 1

- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.107
Reinforcing Steel	LB/LF	24.78

(The above quantities are based on a 2% deck cross slope; railing on low side of deck.)

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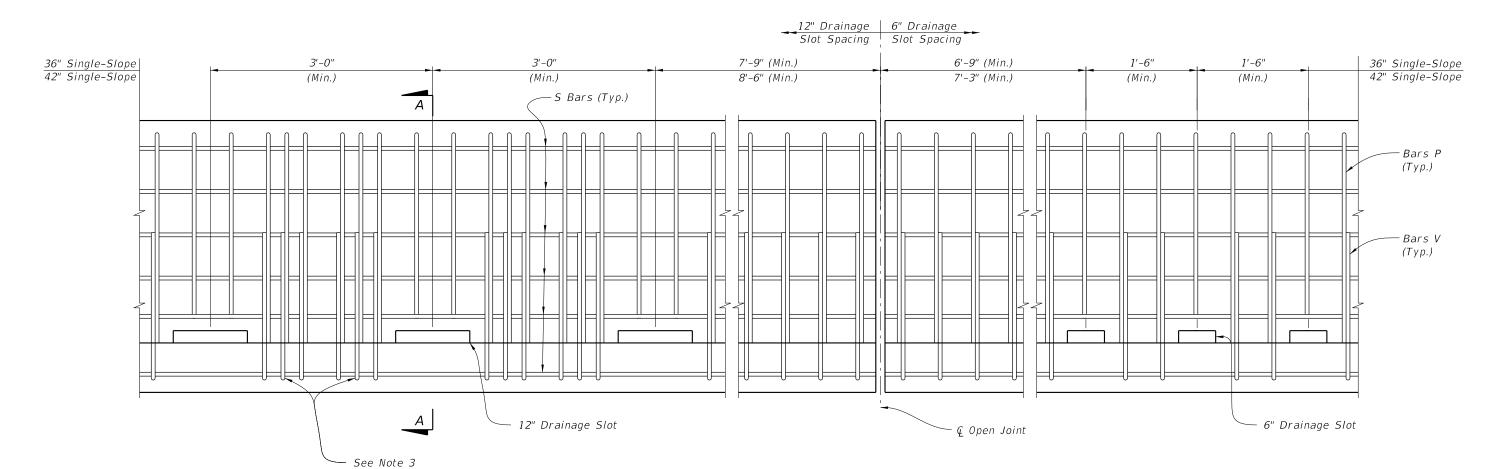


FY 2024-25 STANDARD PLANS

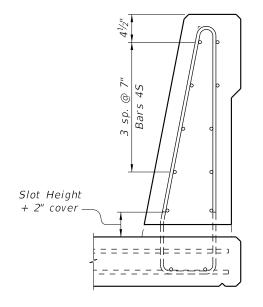
INDEX *521-427*

Field Cut and Lapped

SHEET







SECTION A-A 36" Single-Slope Shown Other traffic railings similar

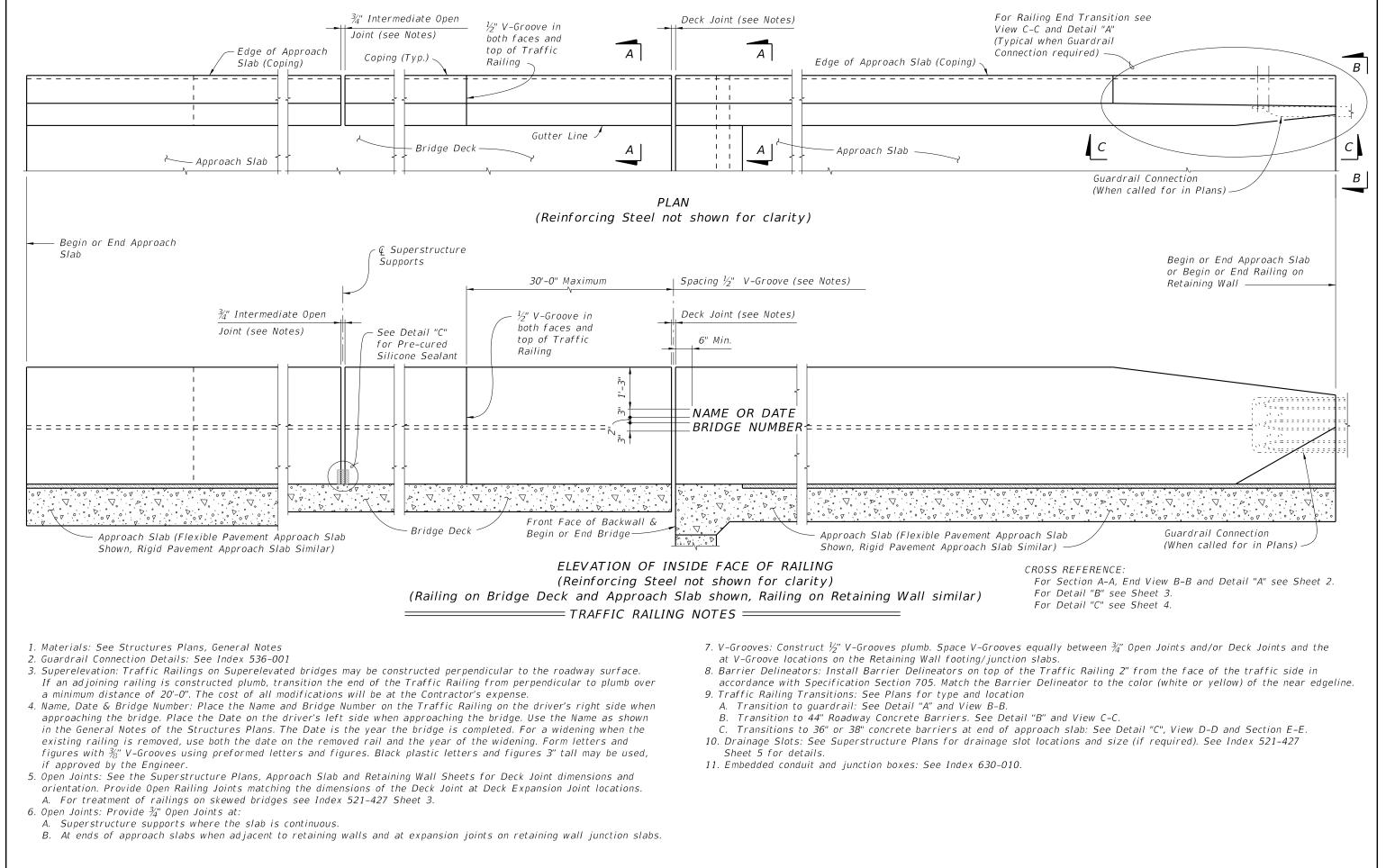
DRAINAGE SLOT NOTES:

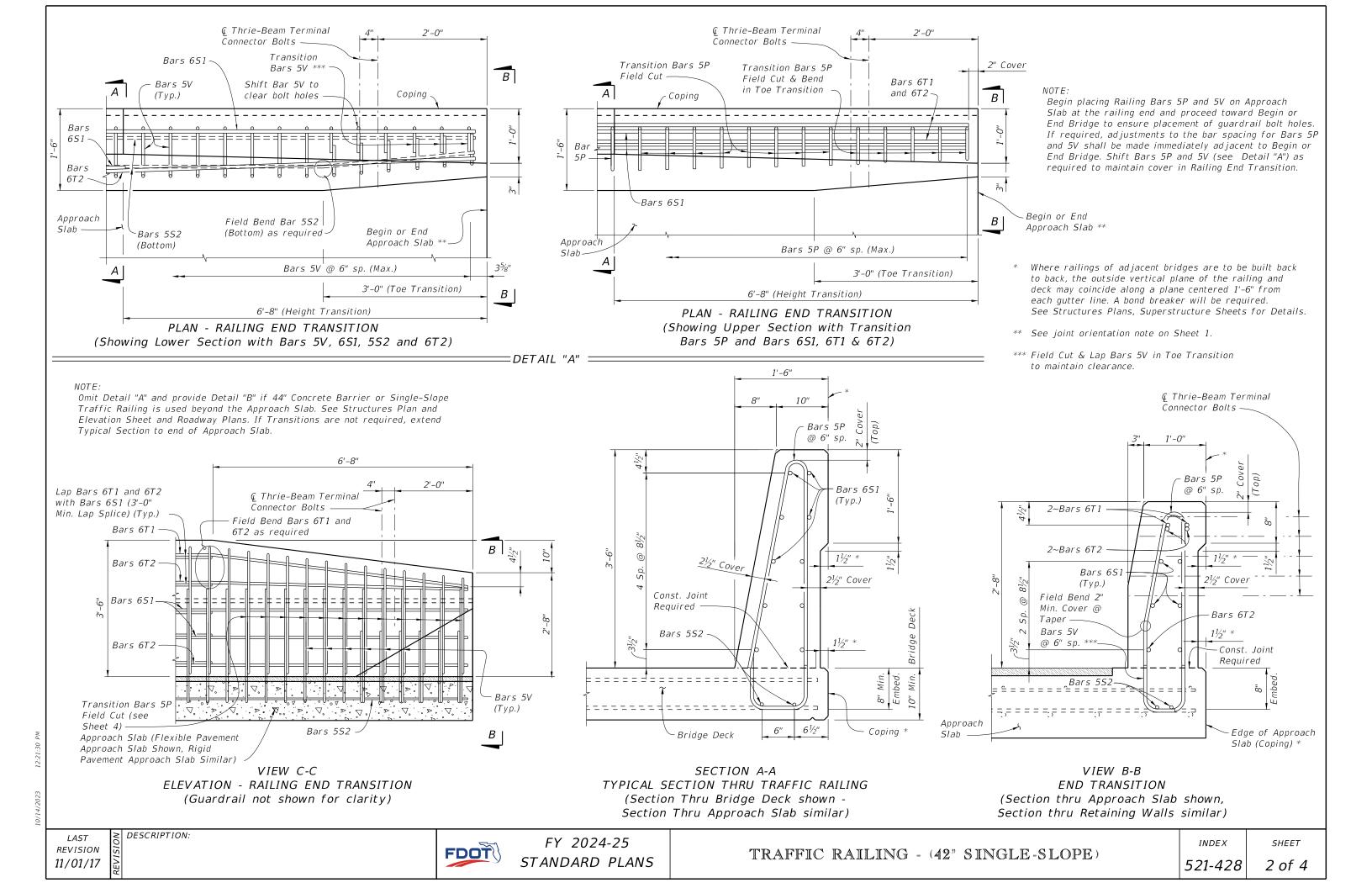
- 1. Use only when required for safety. See Plans for locations and size of drainage slots.
- 2. Maintain 2" minimum cover to all reinforcing. Trim P Bars over drainage slots and raise bottom S bars as necessary to maintain cover.
- 3. For slots greater than 6" in length, add additional vertical bars (V & P) on each side of
- 4. Drainage slot heights are 2" or 3". See the plans for size and location details.

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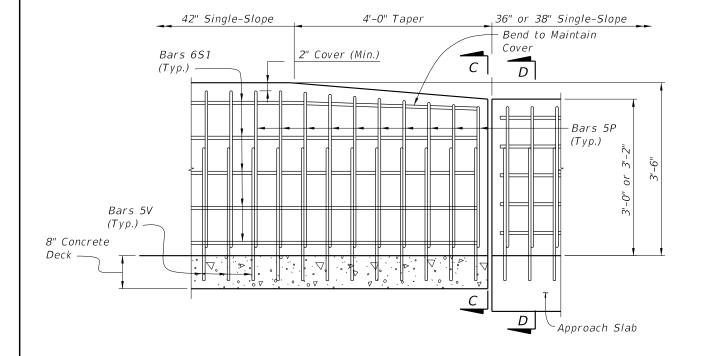
DESCRIPTION:

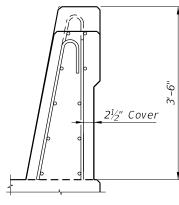
FDOT



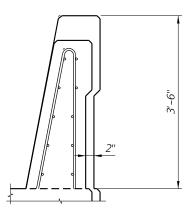


- 1. Provide Detail "B" height transition where 42" Traffic Railings are required on bridge, and 36" or 38" Barriers are shown on approaches. See Structures Plans for coping details.
- 2. Work Detail "B" with Indexes 400-090 or 400-091, 521-427, and 521-610 as necessary.
- 3. Field cut 5P Bars as shown to maintain 2" min. (4" max.) cover at top of traffic railing.





VIEW C-C RAILING HEIGHT TRANSITION (Begin/End of Bridge) (Bars 5V not shown for clarity)



SECTION D-D (Index 400-091 Shown, 400-090 Similar) (Index 521-427 Bars 4V not shown for Clarity)

DETAIL "B"

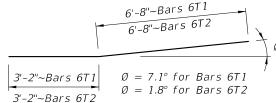
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL		
MARK	SIZE	LENGTH
Р	5	7'-0"
<i>51</i>	6	As Reqd.
52	5	As Reqd.
T1 & T2	6	10'-0"
V	5	5'-9"

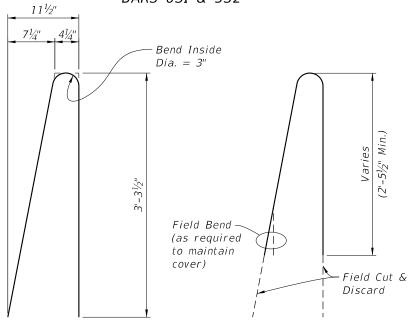
Length as Required

ROADWAY	LOW GUTTER	HIGH GUTTER
CROSS-SLOPE	ØВ	ØВ
0% to 2%	101°	101°
2% to 6%	98°	104°
6% to 10%	95°	107°

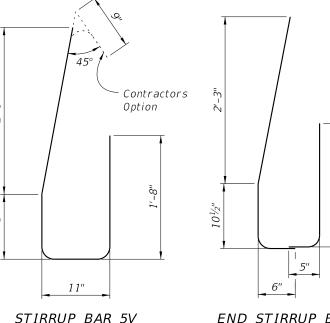
ØA and ØB shall be 90° if Contractor elects to place Railing perpendicular to the Deck.



BARS 651 & 552



TRANSITION BARS 6T1 & 6T2 (2~Bars 6T1 & 3~Bars 6T2 required per Railing End Transition)



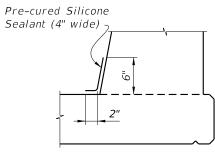
STIRRUP BAR 5P

TRANSITION STIRRUP BAR 5P To Be Field Cut (10 of each required per Railing End Transition)

REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 6S1 may be continuous or spliced at the construction joints. Lap splices for Bars 6S1 and 5S2 shall be a minimum of 3'-0" and 2'-2", respectively.
- 4. The Contractor may utilize deformed WWR when approved by the Engineer. WWR must meet the requirements of Specification Section 931.

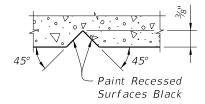
END STIRRUP BAR 5V To Be Field Cut and Lapped



DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.143	
Reinforcing Steel	LB/LF	39.34	

The estimated railing quantities are based on a 2% deck cross slope; railing on low side of deck.

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CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

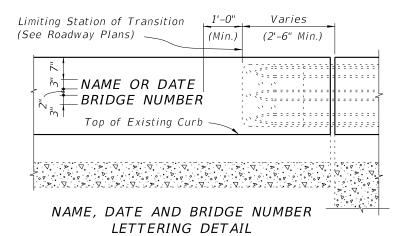
EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment). BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or $\frac{3}{8}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures. ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

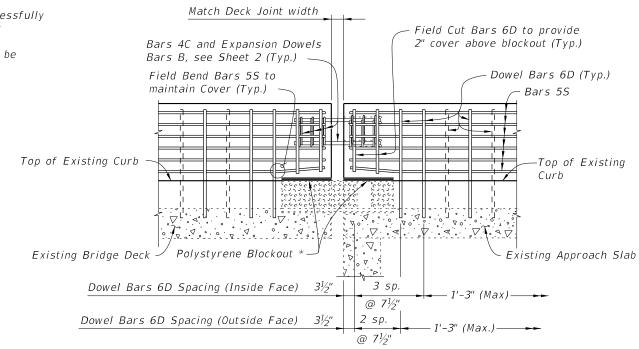
BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing 2" from the face on the traffic side in accordance with Specification Section 705. Match the Barrier Delineator color (white or yellow) to the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) includes all materials and labor required to construct the railing and incidental work as required for transition blocks, curbs, spread footing approaches, and Barrier Delineators.



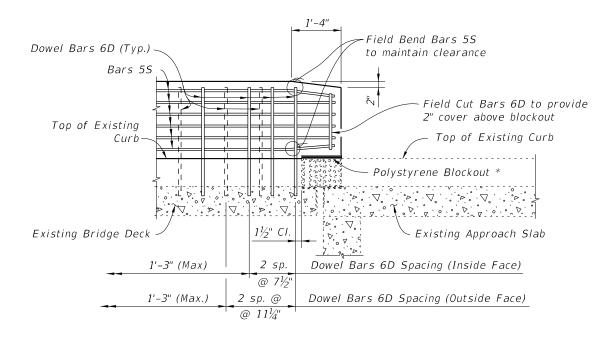
ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
		9" Curb	Increment
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index 521-484, Sheet 4 for Spread Footing Approach Quantities.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT - SCHEMES 2 THRU 5 (Begin or End Bridge Shown, Intermediate Joints Similar)

* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEME 1 (Guardrail Transition not shown for clarity)

REVISION 07/01/19

DESCRIPTION:

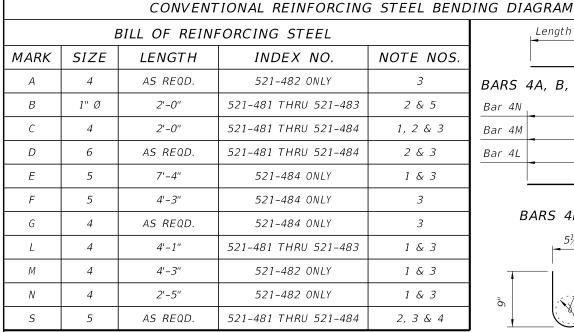
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FY 2024-25 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

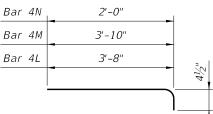
INDEX *521-480*

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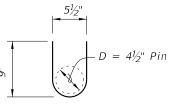


Length as Required

BARS 4A, B, 6D, 5F, 4G & 5S



BARS 4L, 4M & 4N



BARS 4C (12 required per open joint)

REINFORCING STEEL NOTES:

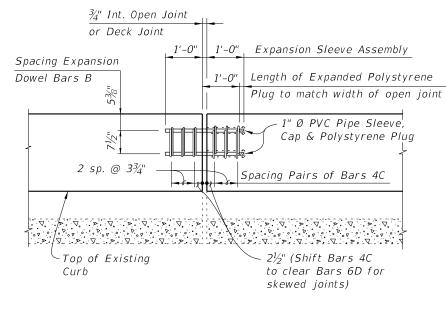
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.



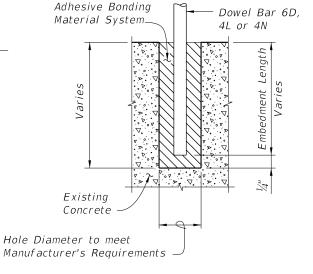
BARS 5E

BARS 5E (Typical Section) (Tapered End Transition)

10"



OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



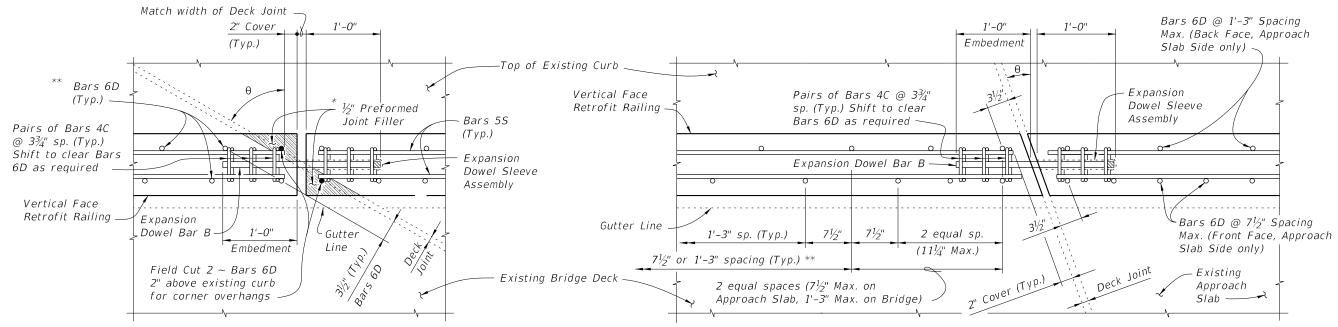
DOWEL DETAIL

Dowel Installation Notes:

- 1. Shift dowel holes to clear if the existing reinforcement is encountered.
- 2. See Index 521-481 thru 521-484 for required embedment length of Bars 6D, 4L or 4N.

* $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.

** See Index 521-481 thru 521-484 for spacing of Bars 6D.



PARTIAL PLAN OF RAILING (SKEW ANGLE 0 GREATER THAN 20°) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

PARTIAL PLAN OF RAILING (SKEW ANGLE $\theta = 20^{\circ}$ OR LESS) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

SKEW DETAIL

REVISION 11/01/16

DESCRIPTION:

FDOT

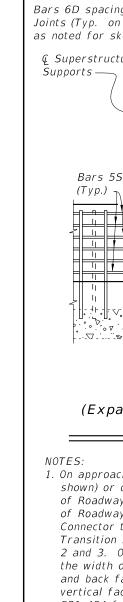
FY 2024-25 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) TYPICAL DETAILS & NOTES

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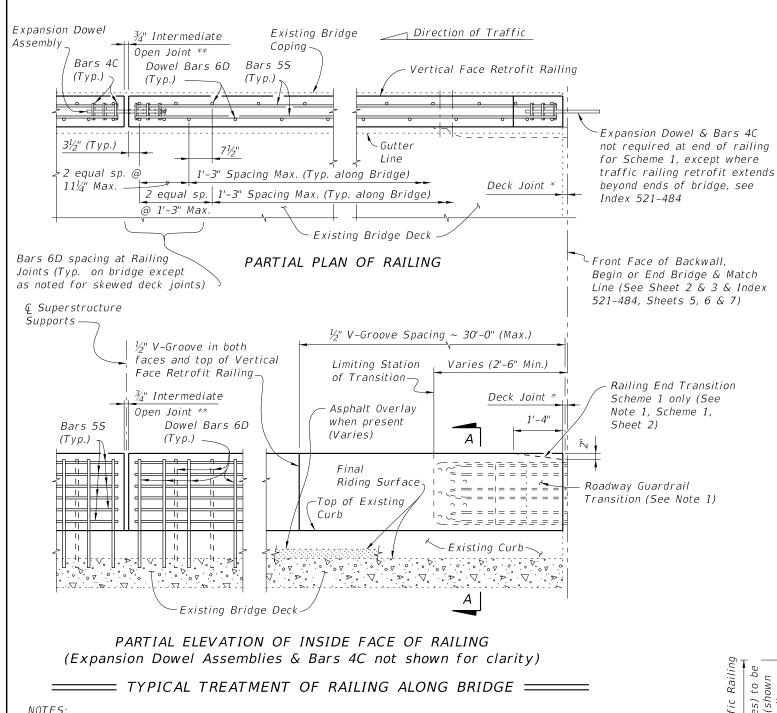
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LAST

REVISION

07/01/13

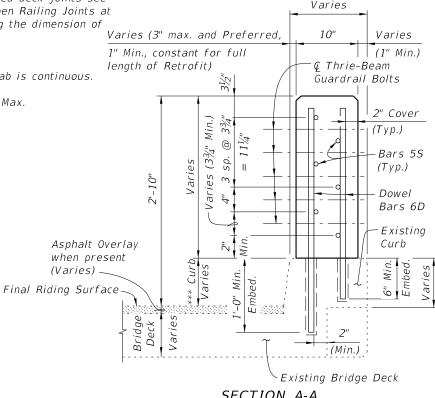


- 1. On approach end provide a Roadway Guardrail Transition, Index 536-002 (as shown) or other site specific treatment. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is on the bridge, attach Thrie Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is along the Wing Wall, see Schemes 2 or 3, Index 521-481, Sheet 2 and 3. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing. For treatment of trailing end see Roadway Plans. If vertical face retrofit extends beyond bridge and approach slab ends, see Index 521-484 for treatment and Details.
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.

DESCRIPTION:

3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off 1" below existing concrete and grouted over.

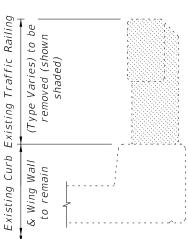
- * Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index 521-480. Provide open Railing Joints at Deck Expansion Joint locations matching the dimension of the Deck Joint.
- ** Provide 3/4" Intermediate Open Joints at: (1) - Superstructure supports where slab is continuous
- *** Curb heights vary from 5" Min. to 1'-2" Max.



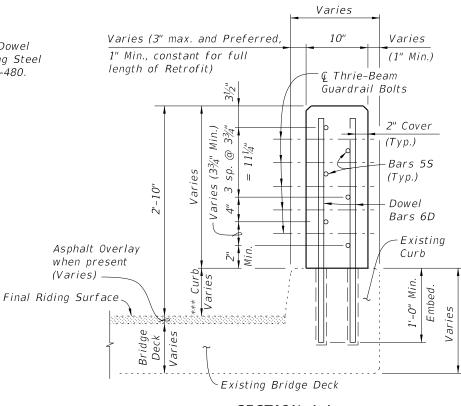
SECTION A-A TYPICAL SECTION THRU RAILING ON CURB WITH CORBELS

CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagrams see Index 521-480.



TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION A-A TYPICAL SECTION THRU RAILING ON FULL DEPTH CURB (BRIDGE SHOWN, WING WALL SIMILAR)

FDOT

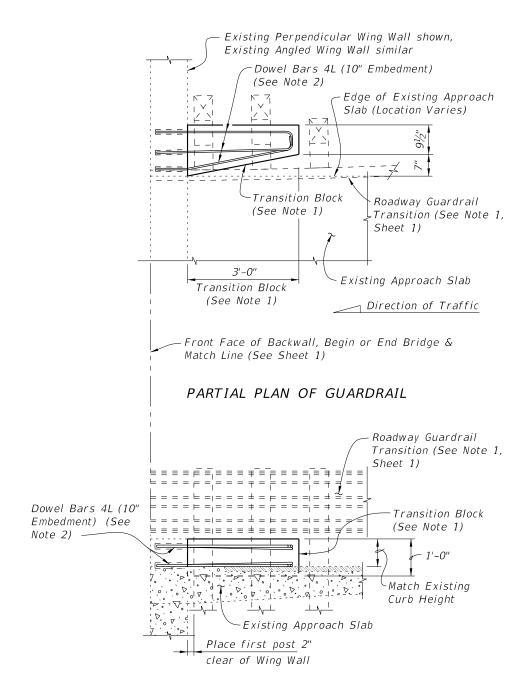
FY 2024-25 STANDARD PLANS TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

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SHEET

NARROW CURB



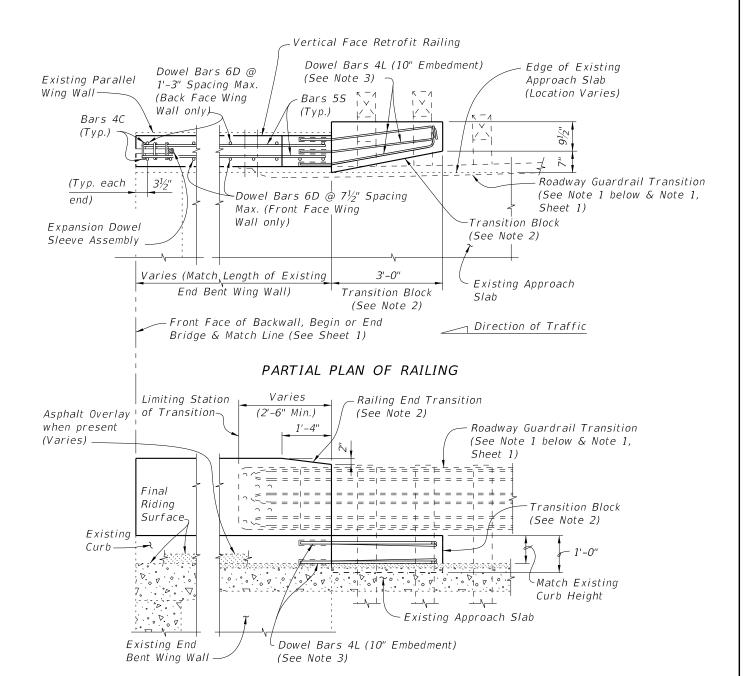


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

_____ SCHEME 1 _____ RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

_____ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Index 521-481, Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> RAILING END TREATMENT FOR FLARED WING WALLS

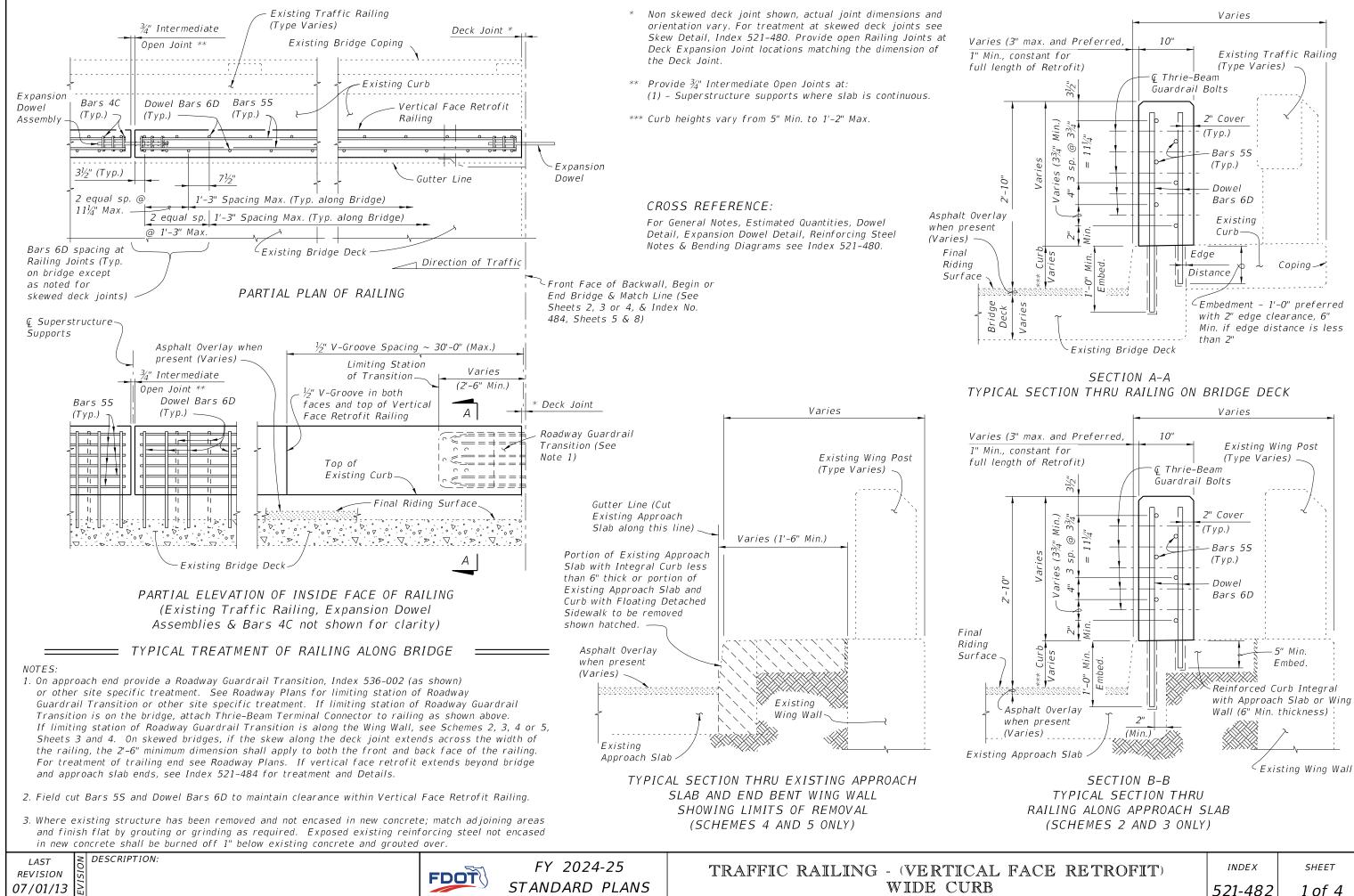
SCHEME 3 NOTE:

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.

LAST REVISION 07/01/07

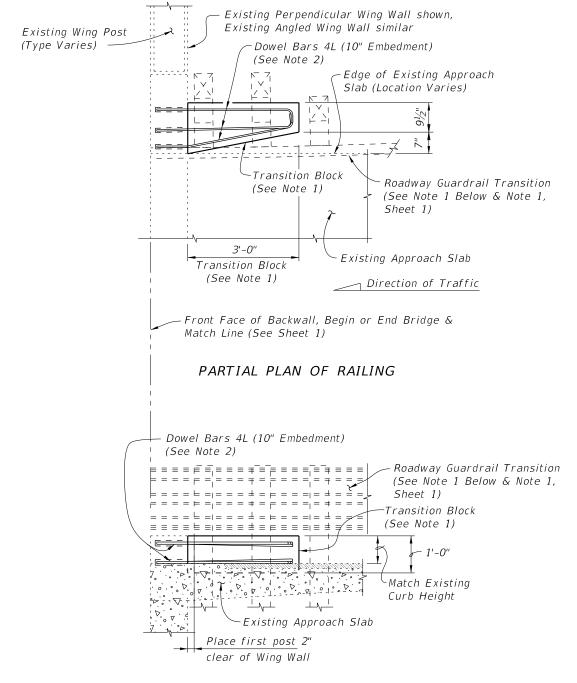
DESCRIPTION:









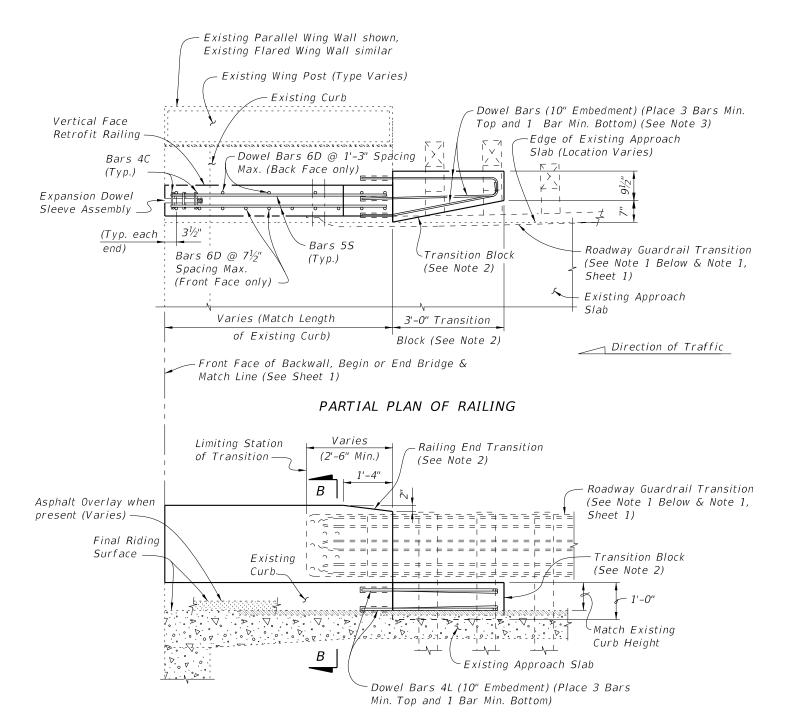


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

===== SCHEME 1 === RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> _______ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

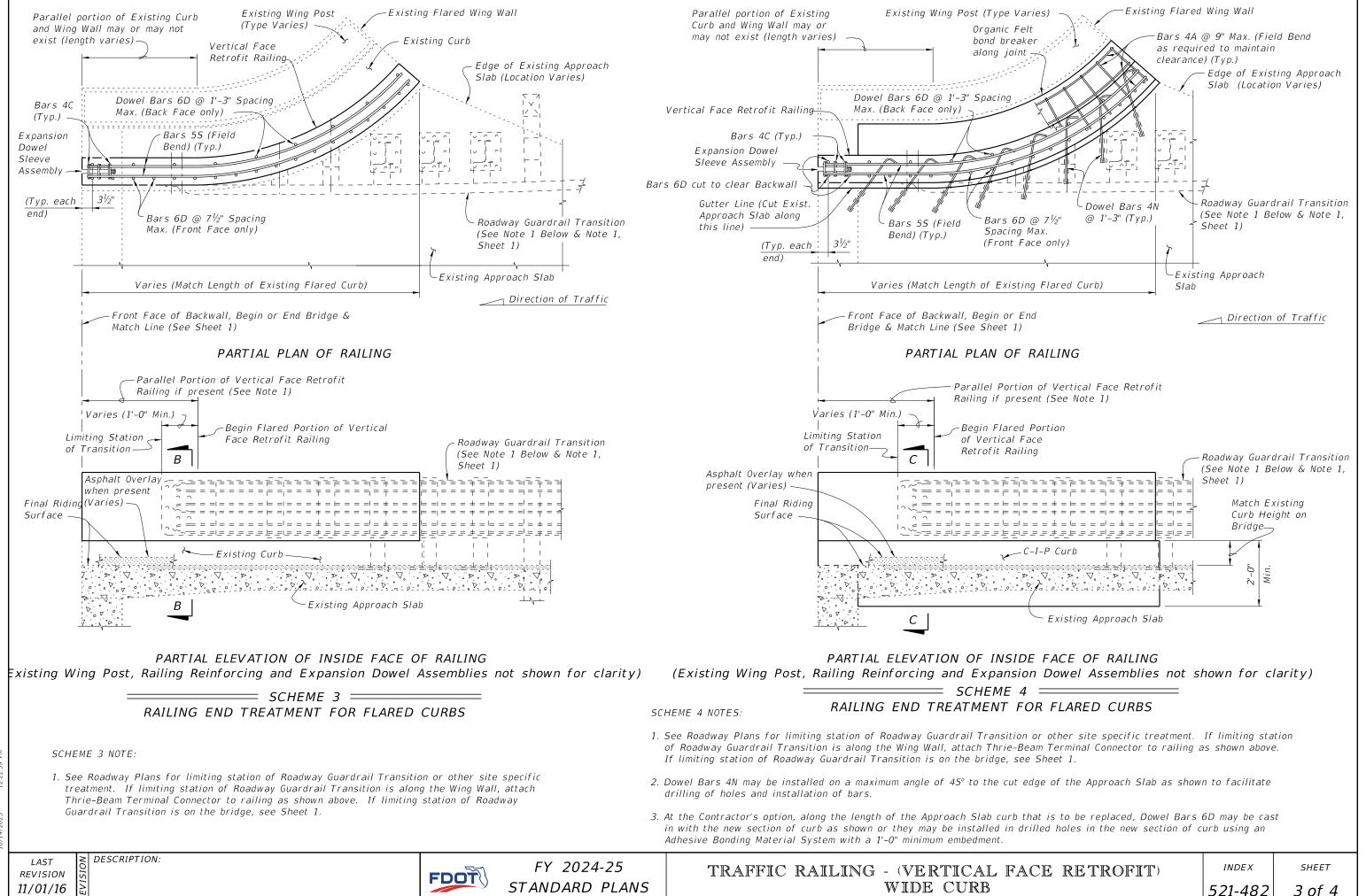
DESCRIPTION: LAST REVISION 07/01/05



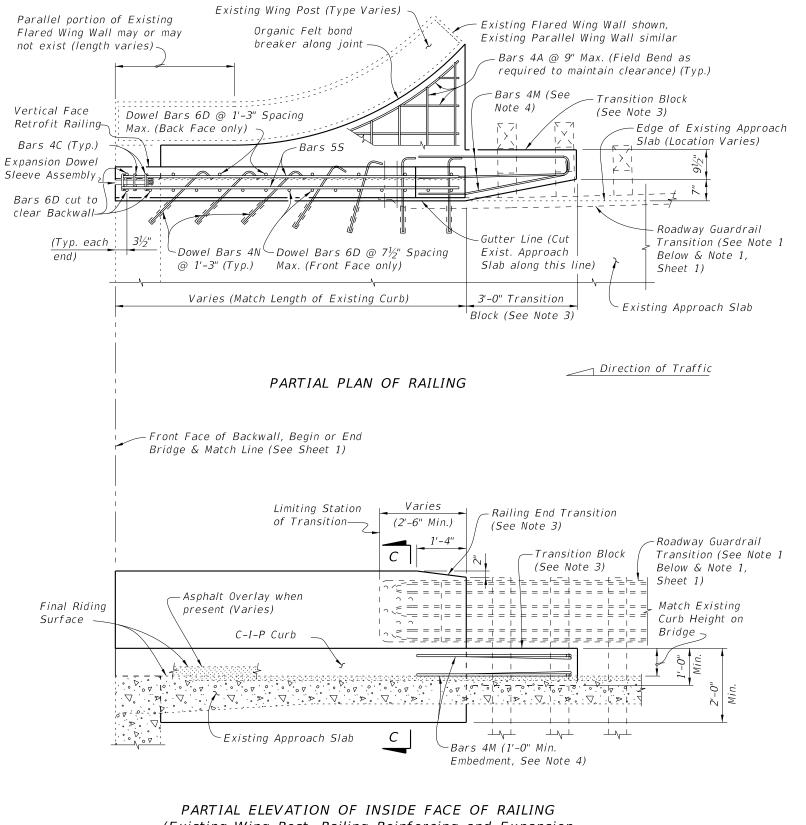
FY 2024-25 STANDARD PLANS TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

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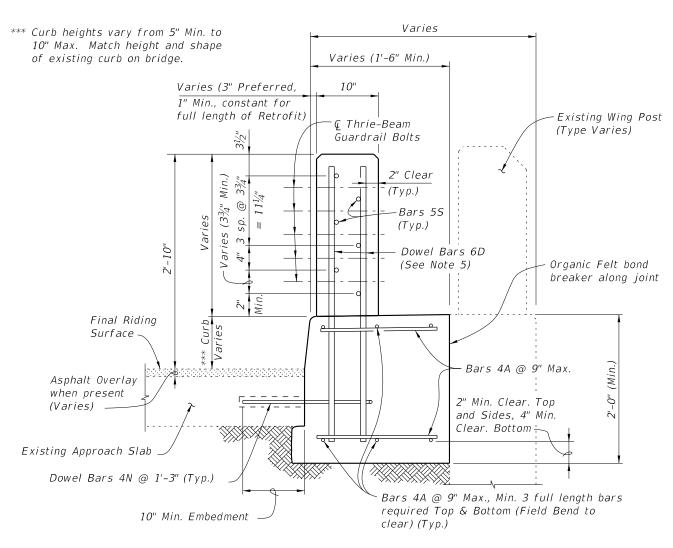


10/14/2023



(Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

______ SCHEME 5 _____ RAILING END TREATMENT FOR PARALLEL CURBS



SECTION C-C TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

SCHEME 5 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.

REVISION 11/01/16

DESCRIPTION:

FDOT

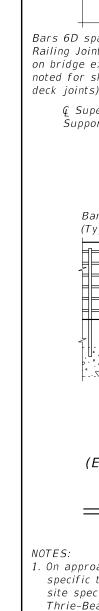
FY 2024-25 STANDARD PLANS

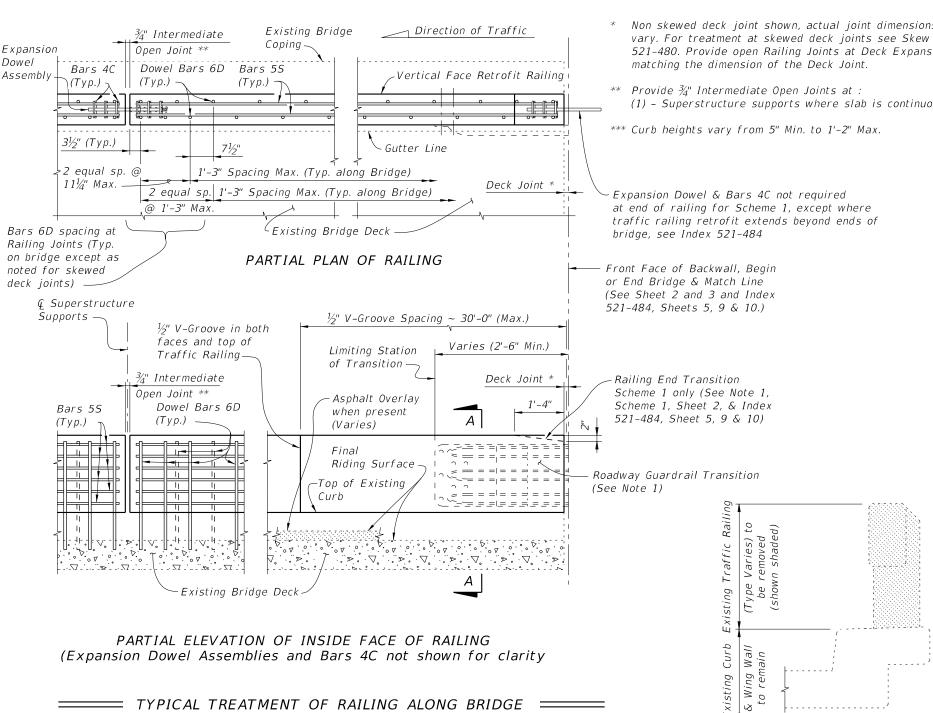
WIDE CURB

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- 1. On approach end provide a Roadway Guardrail Transition, Index 536-002 (as shown) or other site specific treatment. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is on the bridge, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is along the Wing Wall, see Schemes 2 or 3, Sheets 2 & 3. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing. For treatment of trailing end see Roadway Plans. If vertical face retrofit extends beyond bridge and approach slab ends, see Index 521-484 for treatment and Details.
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.

DESCRIPTION:

REVISION

07/01/13

3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off 1" below existing concrete and grouted over.

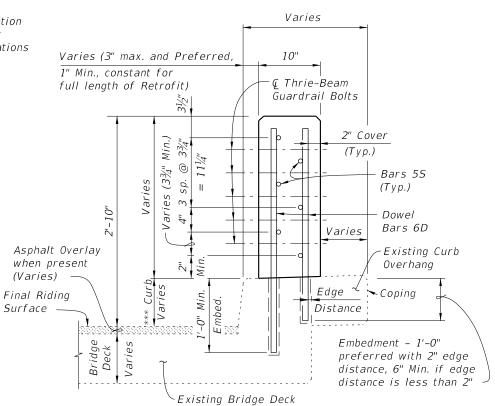
- Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index 521-480. Provide open Railing Joints at Deck Expansion Joint locations
- (1) Superstructure supports where slab is continuous.

Existing

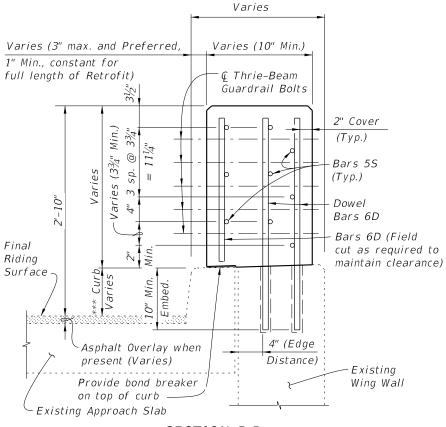
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagram see Index 521-480.



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK



SECTION B-B TYPICAL SECTION THRU RAILING ON WING WALL

FDOT

FY 2024-25 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

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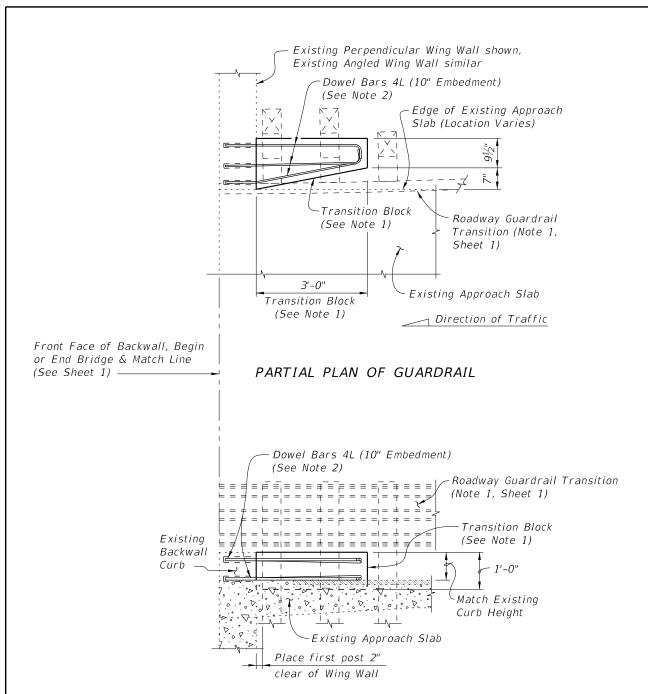
SHEET 1 of 3





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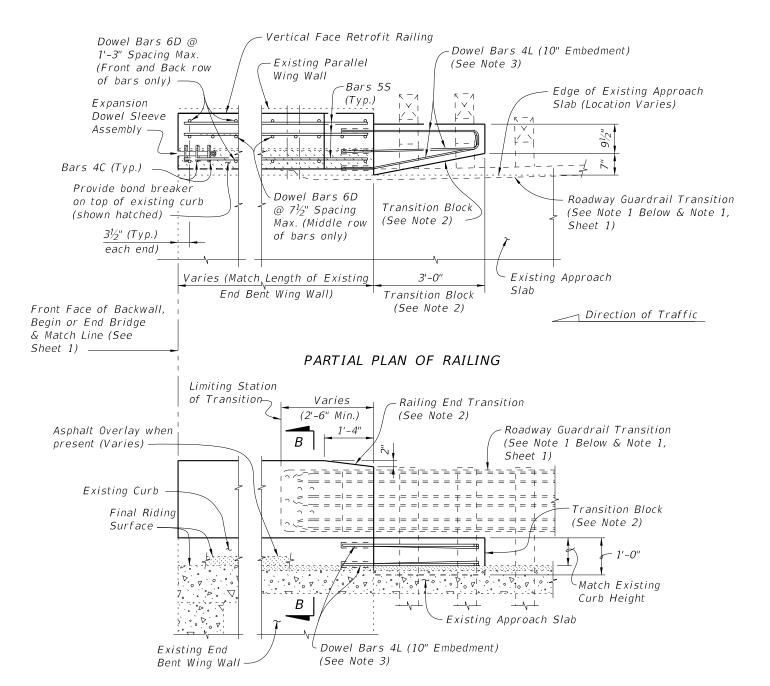
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

DESCRIPTION:

- Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> RAILING END TREATMENT FOR FLARED WING WALLS

REVISION 07/01/07 DESCRIPTION:

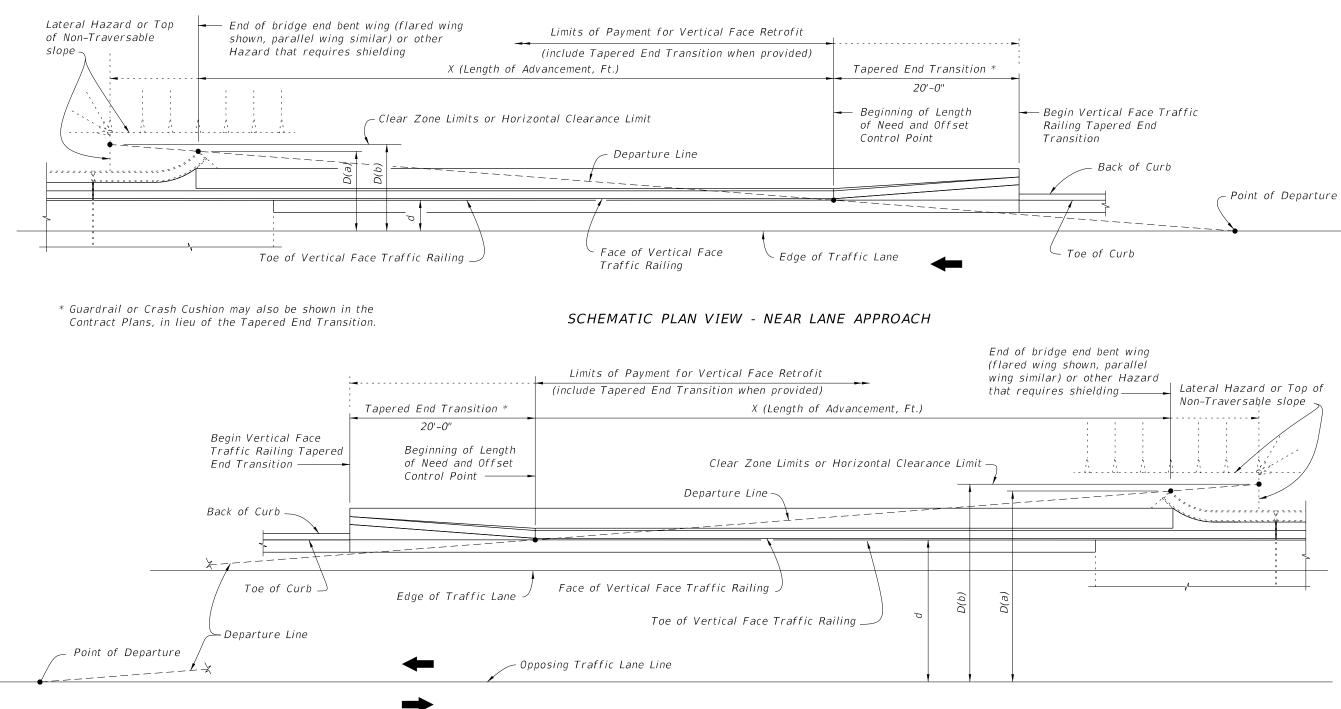
FDOT

or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector

to railing as shown above. If limiting station of Roadway Guardrail

Transition is on the bridge, see Sheet 1.

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) INTERMEDIATE CURB



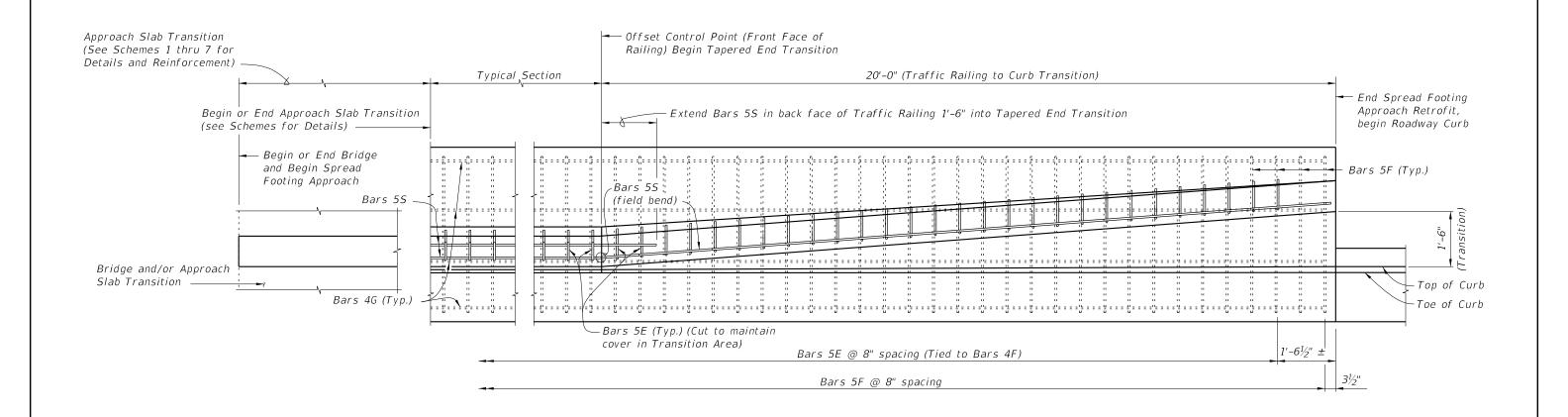
SCHEMATIC PLAN VIEW - OPPOSING LANE APPROACH

CROSS REFERENCES:

For General Notes, Dowel Details, Expansion Dowel Details, Reinforcing Steel Notes and Reinforcing Steel Bending Diagram see Index 521-480.

LAST REVISION 07/01/09 DESCRIPTION:

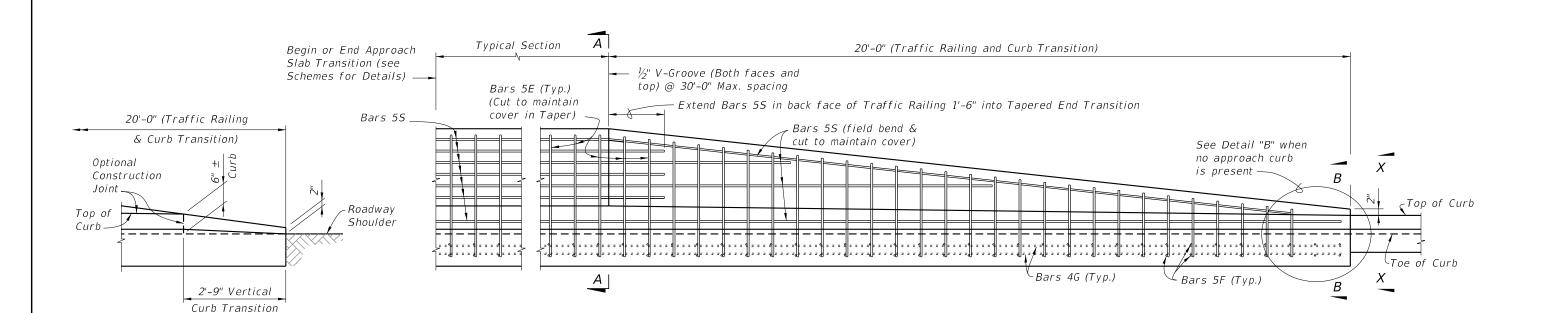
FDOT



PARTIAL PLAN VIEW

PARTIAL ELEVATION VIEW

TAPERED END TRANSITION =



REVISION 07/01/09

DESCRIPTION:

DETAIL "B" TRANSITION TO NON-CURB APPROACH

(Reinforcing Not Shown For Clarity)

FDOT

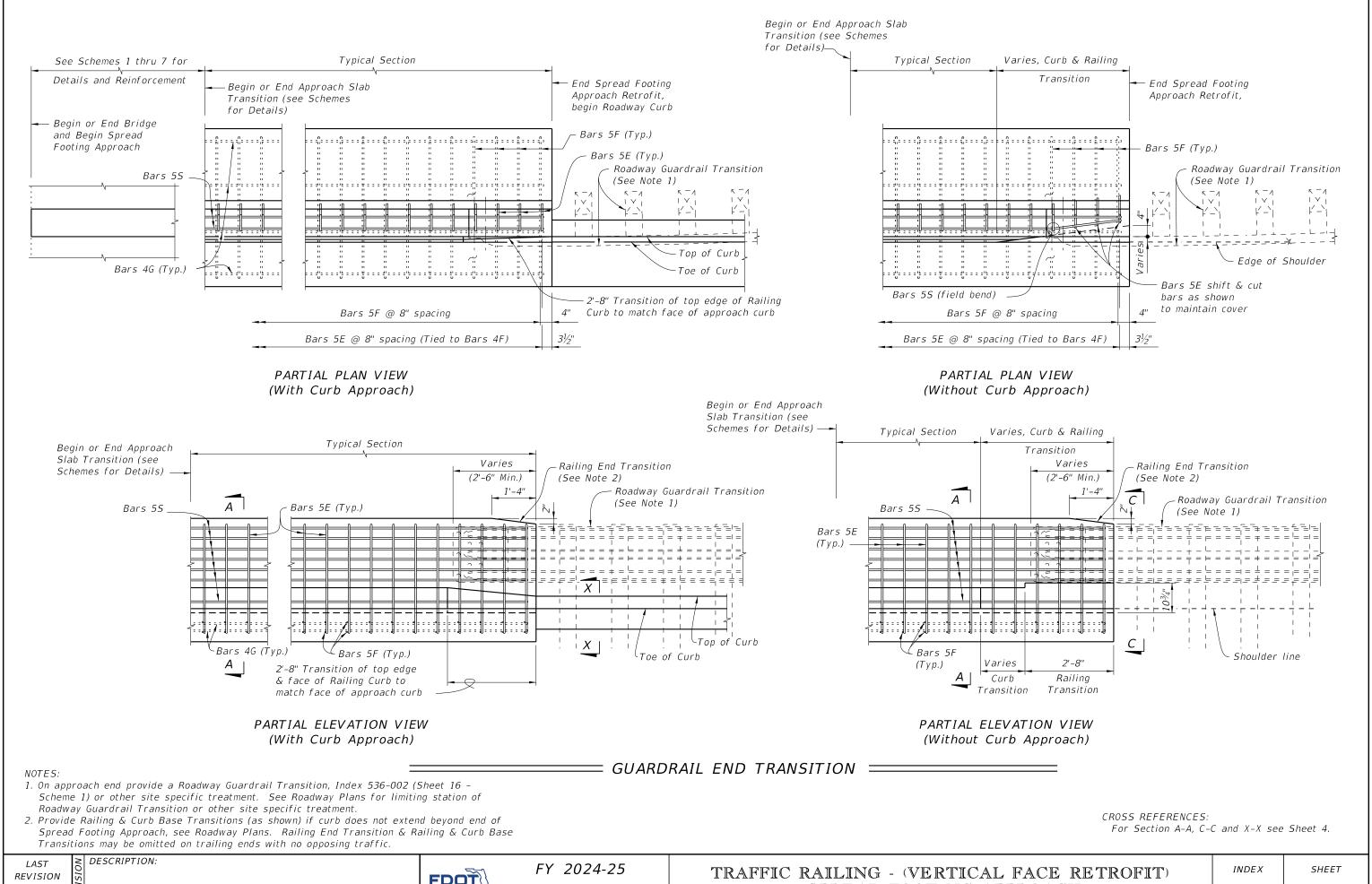
FY 2024-25 STANDARD PLANS TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

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For Section A-A, B-B and X-X see Sheet 4.

CROSS REFERENCES:

SHEET



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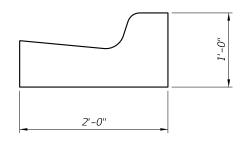
FDOT

STANDARD PLANS

SPREAD FOOTING APPROACH

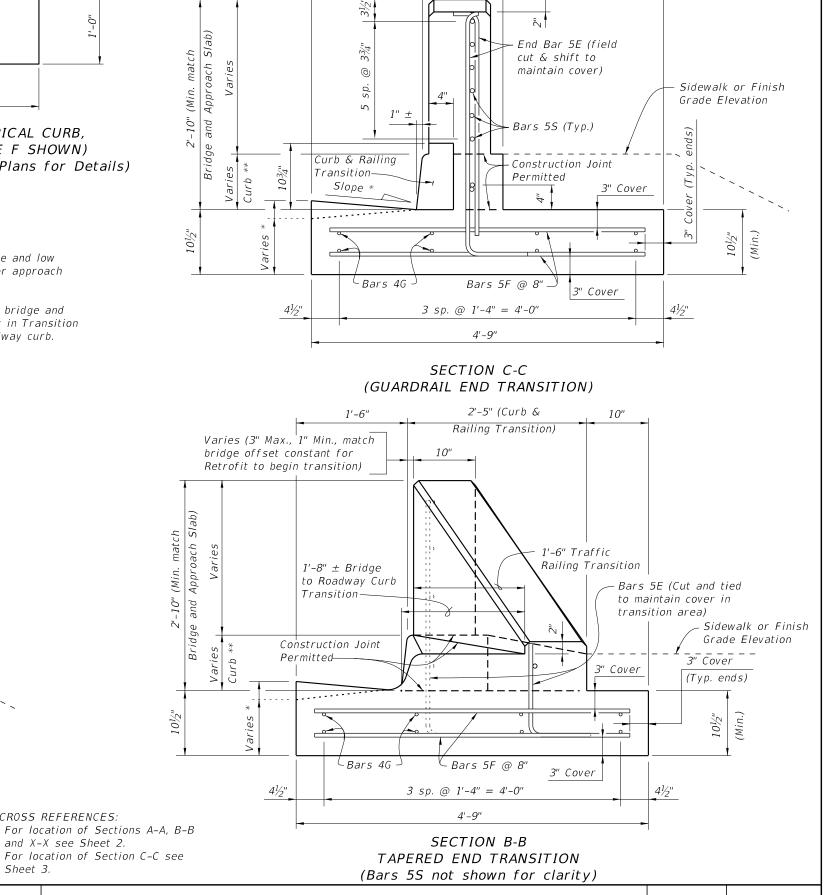
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES			
ITFM	//A/IT	QUANTITY	
	UNIT	9" Curb	
Concrete - Typical Section	CY/Ft.	0.25	
Reinforcing Steel - Typical Section	Lb./Ft.	38	
Concrete – 20'-0" Tapered End Transition plus Footing	CY	4.57 Total	
Reinforcing Steel - 20'-0" Tapered End Transition plus Footing	Lb.	776 Total	

NOTE: Quantities are based on a 9" curb, no curb cross slope.



SECTION X-X (TYPICAL CURB, TYPE VARIES, TYPE F SHOWN) (See Index 520-001 and Plans for Details)

- * Match Cross Slope of high side and low side at begin or end bridge or approach slab.
- ** Match curb height of adjacent bridge and approach slab. Adjust height in Transition area to match adjoining Roadway curb.



2'-2"

Varies (0" to 2")

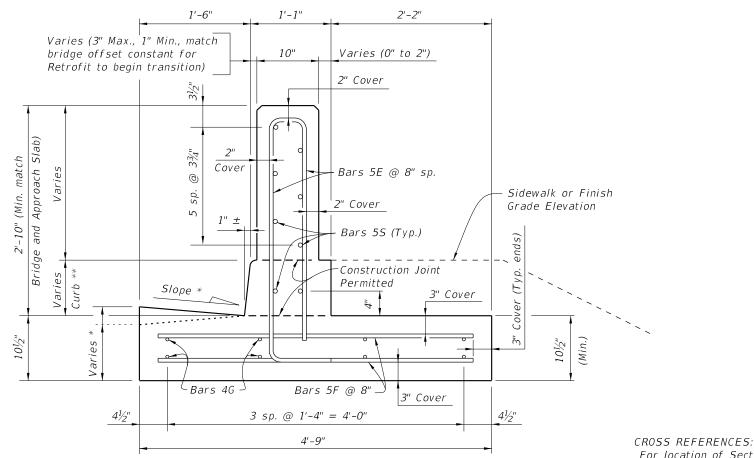
1'-6"

Varies (3" Max., 1" Min., match bridge offset constant for Retrofit

to begin Curb & Railing Transition)

1'-1"

10"



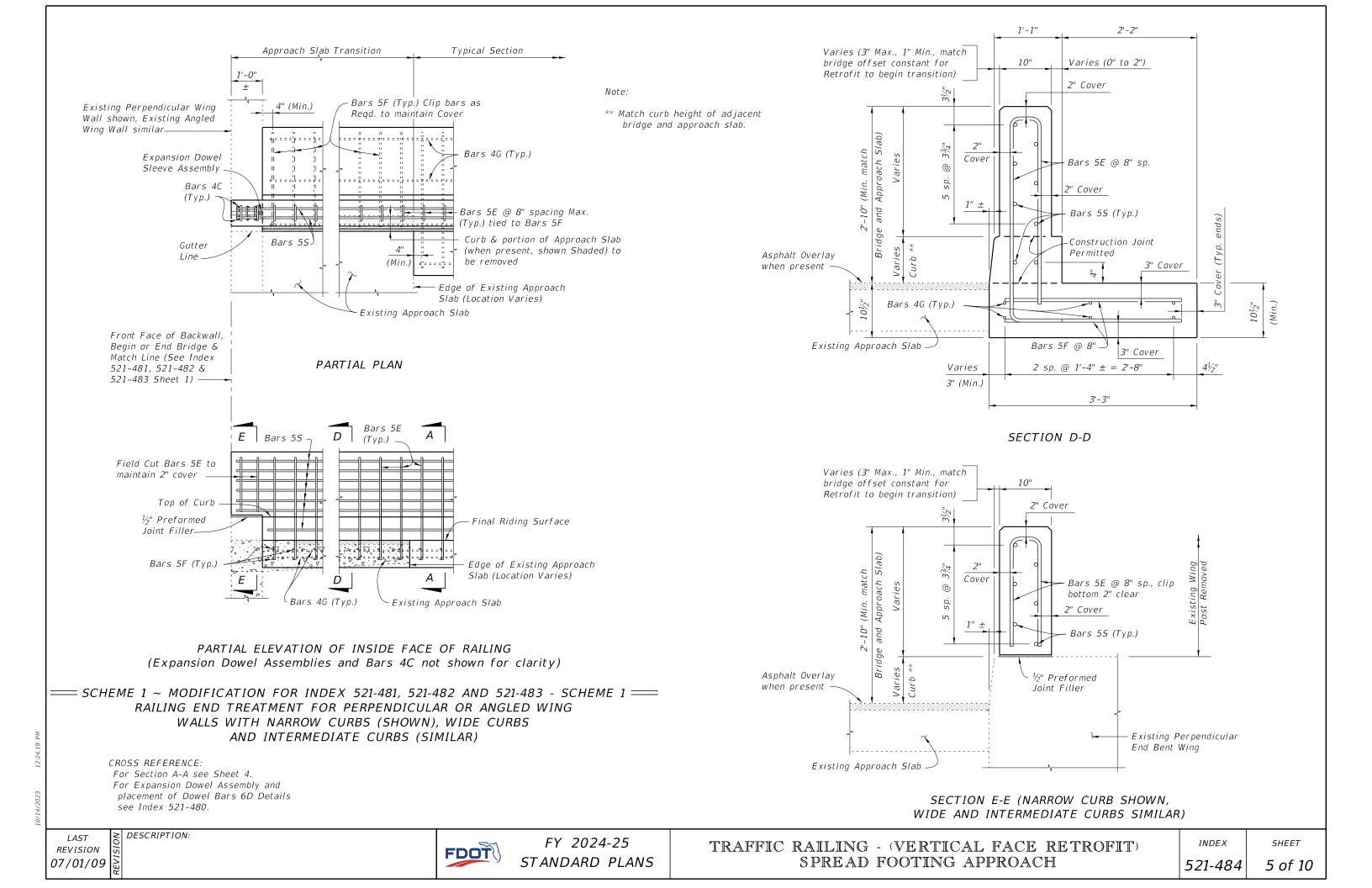
SECTION A-A TYPICAL SECTION (9" Curb shown, 6" Curb similar)

FY 2024-25

and X-X see Sheet 2.

Sheet 3.

DESCRIPTION:

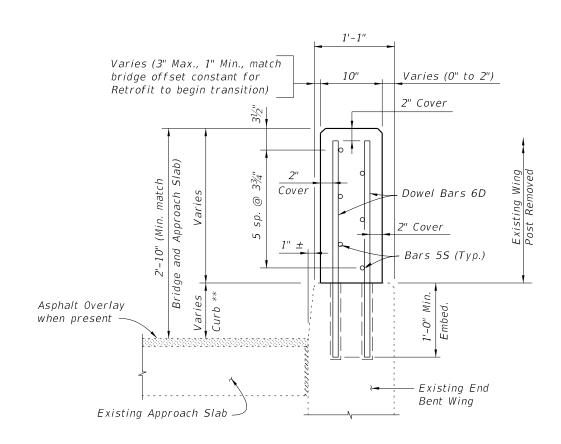


PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

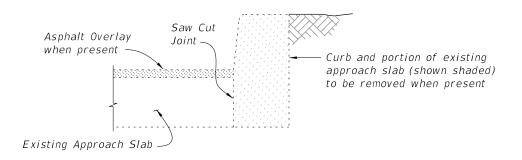
SCHEME 2 ~ MODIFICATION FOR INDEX 521-481 - SCHEME 2 ===== RAILING END TREATMENT FOR PARALLEL WING WALLS WITH NARROW CURBS

DESCRIPTION:

1. Remove existing concrete along saw cut joints. Existing reinforcing steel may be cut at joint or extended into new concrete. Exposed existing reinforcing not encased in new concrete shall be removed 1" below existing concrete surface and grouted over.



SECTION F-F

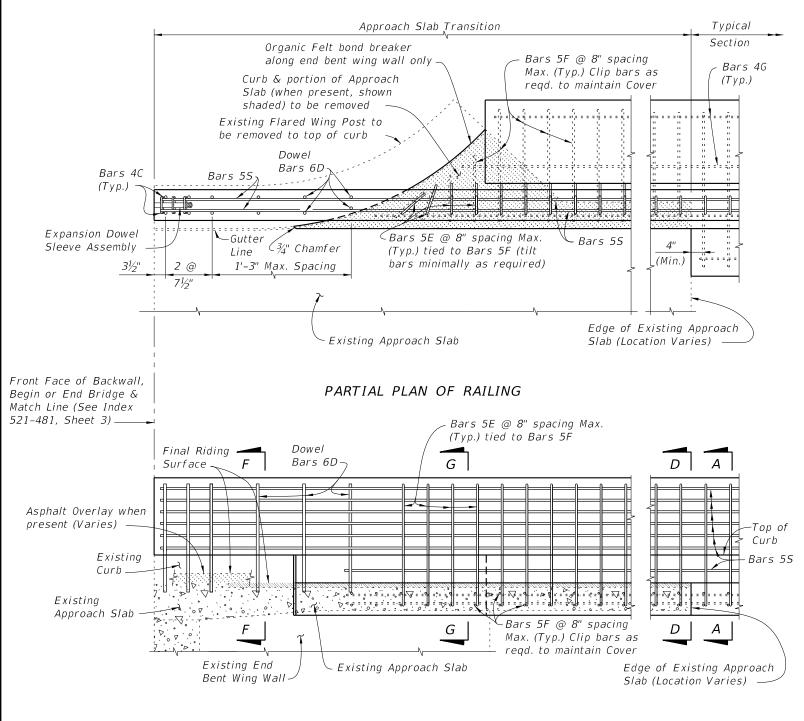


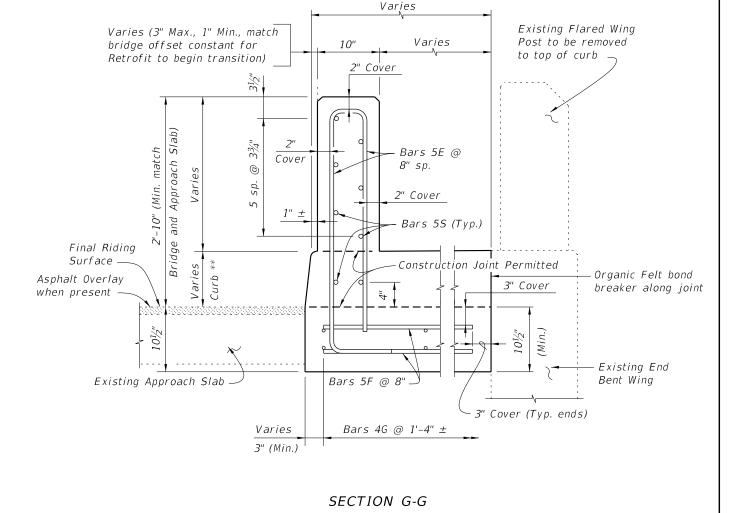
SECTION THRU EXISTING CURB AND APPROACH SLAB TO BE REMOVED (Free Standing Curb Similar)

CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

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** Match curb height at adjoining existing end bent wing.

PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 3 ~ MODIFICATION FOR INDEX 521-481 SCHEME 3 == RAILING END TREATMENT FOR FLARED WING WALLS WITH NARROW CURBS

CROSS REFERENCES:

For Section A-A see Sheet 4.

For Section D-D see Sheet 5.

For Section F-F see Sheet 6.

For Expansion Dowel Assemblies Details and

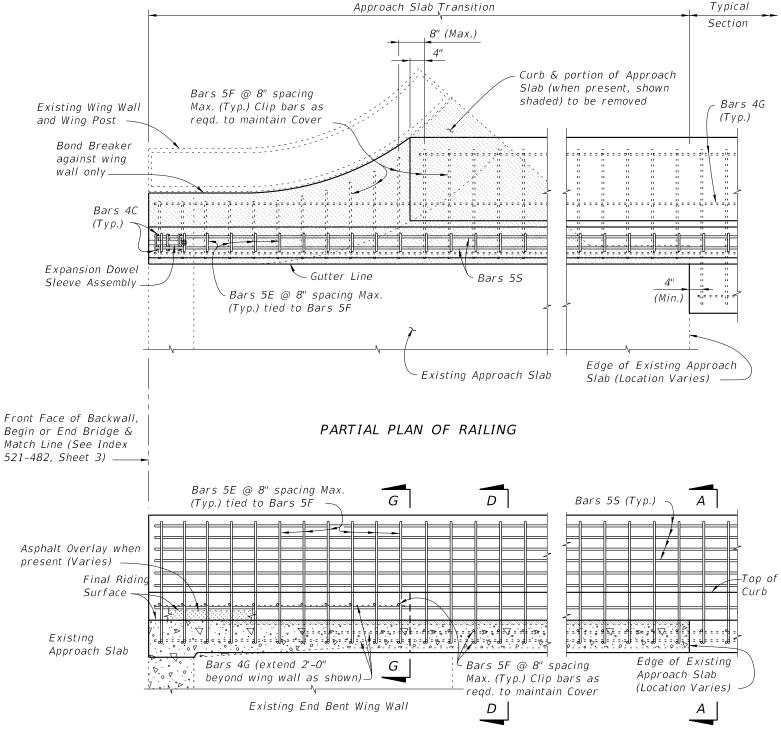
placement of Dowel Bars 6D see Index 521-480.

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DESCRIPTION:

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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Expansion Dowel Assemblies and Bars 4C not shown for clarity)

=== SCHEME 5 ~ MODIFICATION FOR INDEX 521-482 SCHEME 3 AND 4 ==== RAILING END TREATMENT FOR PARALLEL CURBS AND FLARED WING WALLS WITH WIDE CURBS

For Section A-A see Sheet 4

For Section D-D see Sheet 5.

For Expansion Dowel Assemblies Details

see Index 521-480.

REVISION 07/01/09 DESCRIPTION:

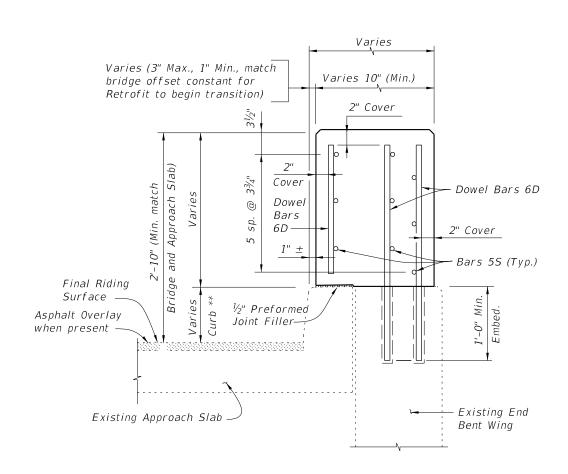
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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 6 ~ MODIFICATION FOR INDEX 521-483 SCHEME 2 ===== RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH INTERMEDIATE CURBS



SECTION H-H

** Match curb height at adjoining existing end bent wing.

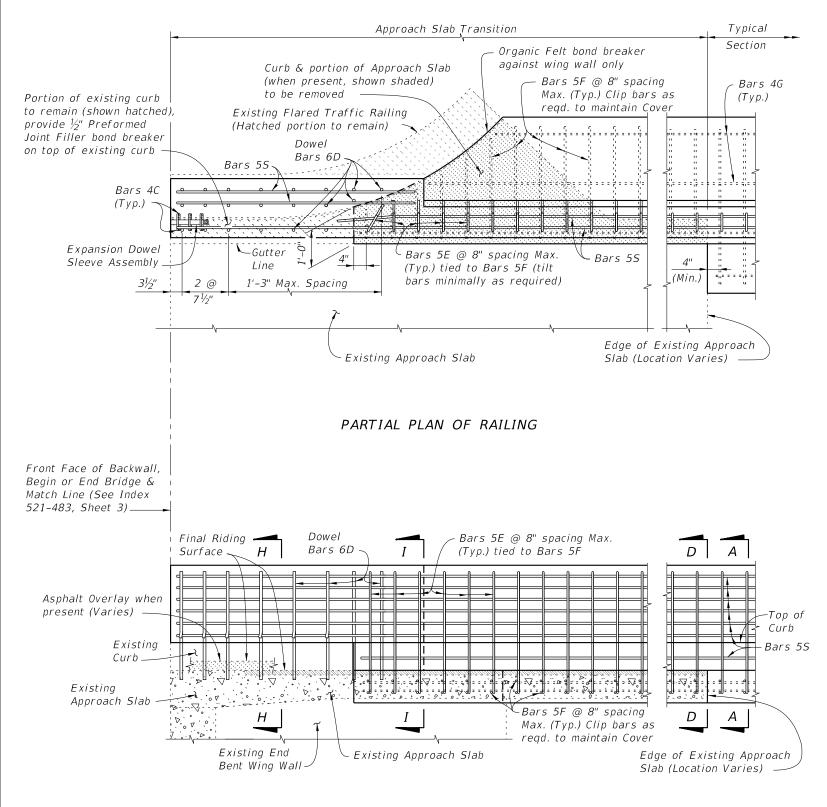
CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

REVISION 07/01/09

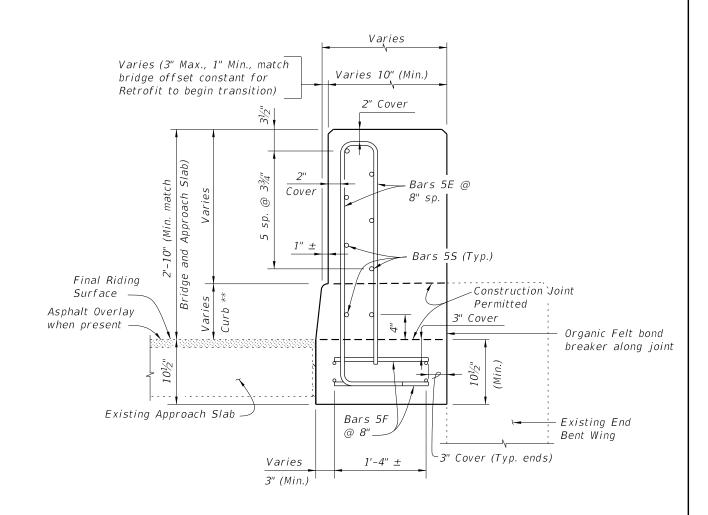
DESCRIPTION:

FDOT



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 7 ~ MODIFICATION FOR INDEX 521-483 SCHEME 3 === RAILING END TREATMENT FOR PARALLEL CURBS AND FLARED WING WALLS WITH INTERMEDIATE CURBS



SECTION I-I

Note: ** Match curb height at adjoining existing end bent wing.

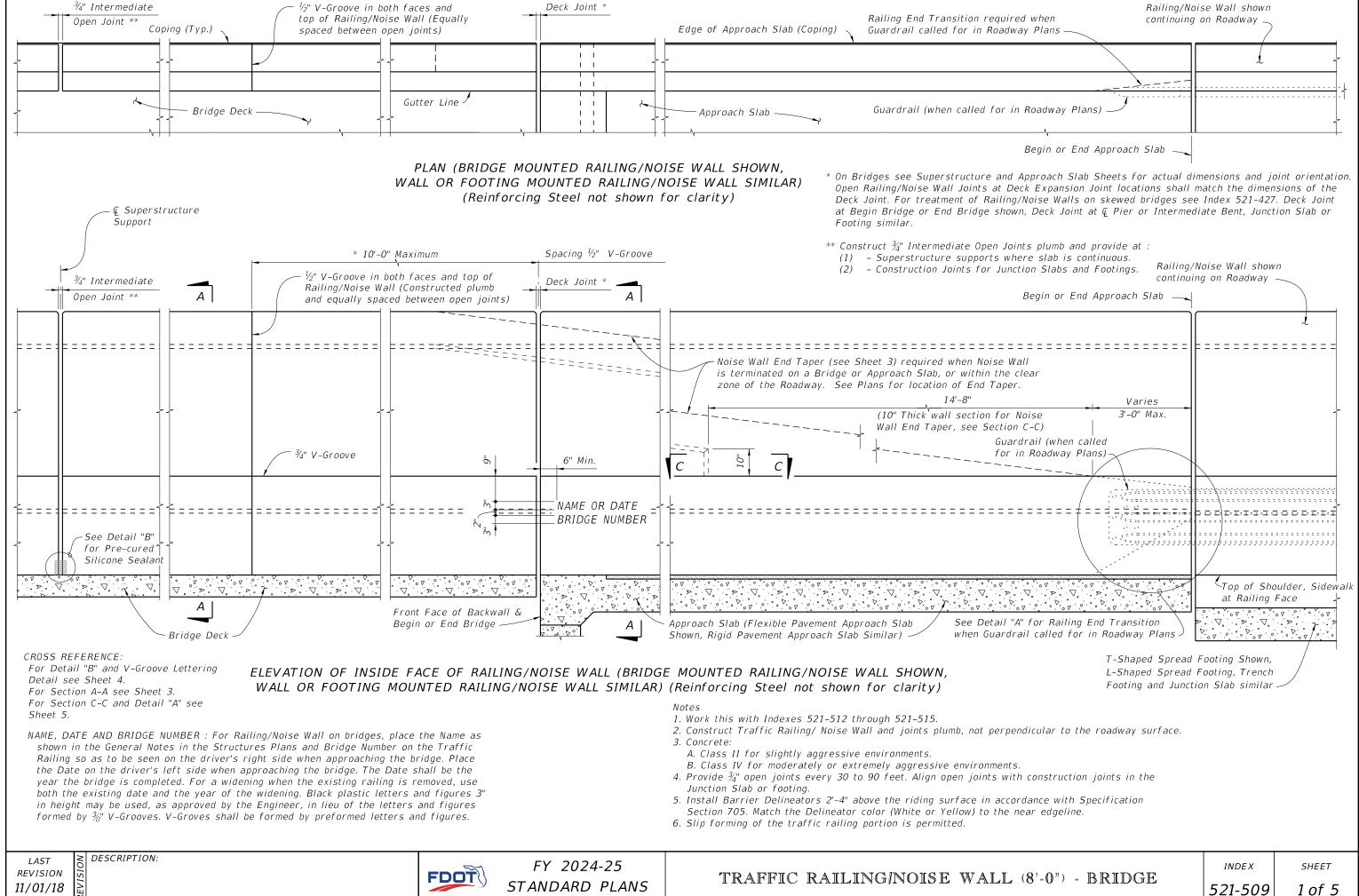
> CROSS REFERENCES: For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section H-H see Sheet 9. For Expansion Dowel Assemblies and placement of Dowel Bars 6D Details see Index 521-480.

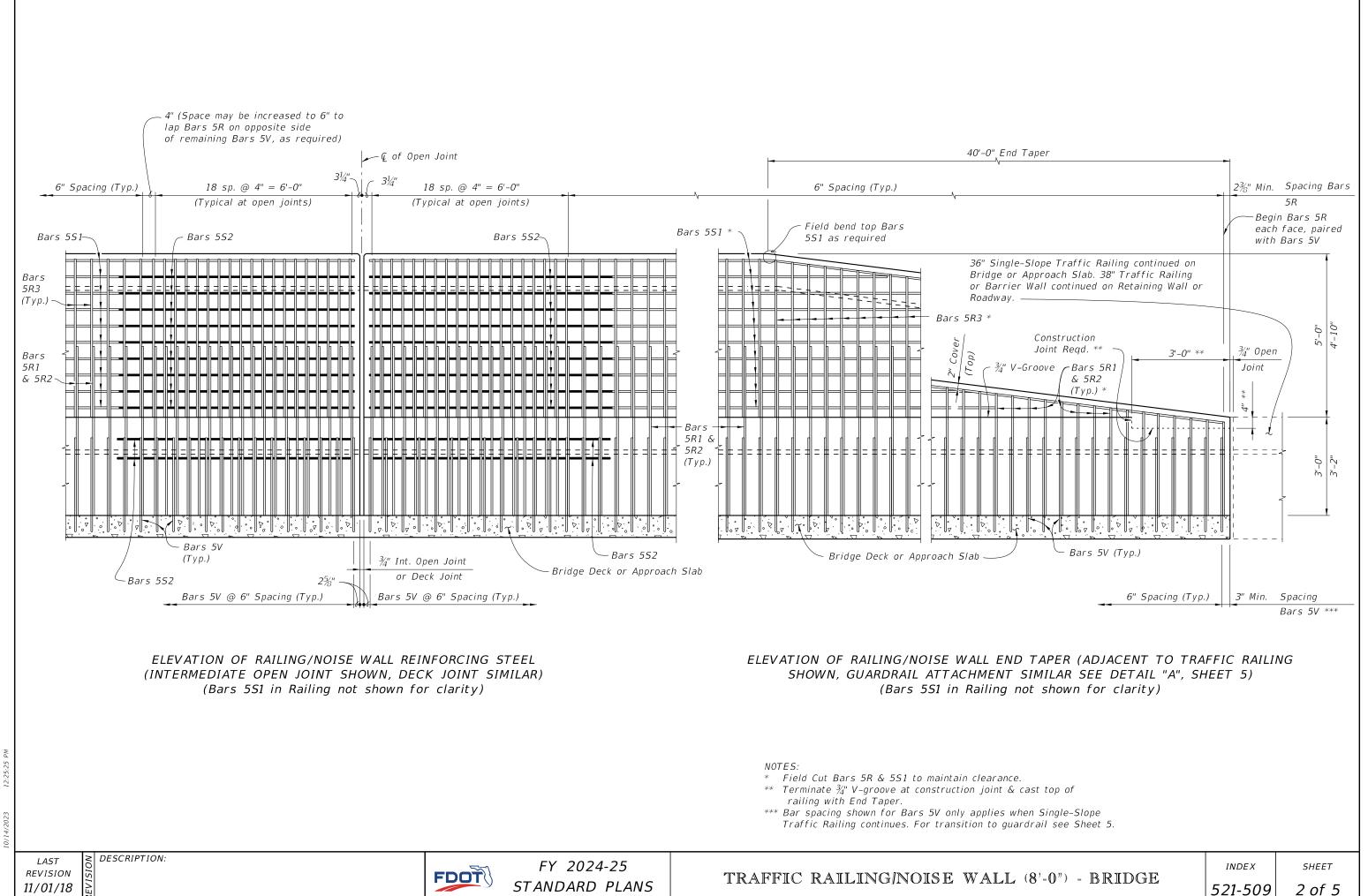
REVISION 11/01/16

DESCRIPTION:

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FY 2024-25 STANDARD PLANS

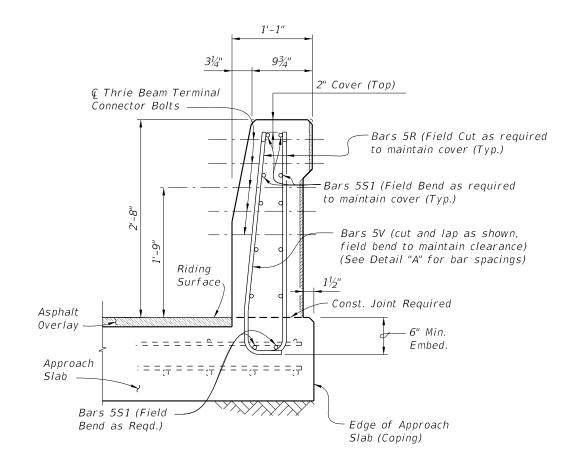




SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING/NOISE WALL (Section Thru Bridge Deck Shown, Section Thru Approach Slab Similar)

CROSS REFERENCE: For locations of Section A-A see Sheet 1. For location of View B-B, see Sheet 5.

1. Bottom Bars 5S1 shown are part of the Traffic Railing/Noise Wall reinforcing. See Superstructure Sheets in the Plans for additional Bridge Deck Reinforcing.



VIEW B-B END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT AT END OF APPROACH SLAB (Flexible Pavement Approach Slab Shown, Rigid Pavement Approach Slab Similar)

REVISION 11/01/18

DESCRIPTION:

FDOT

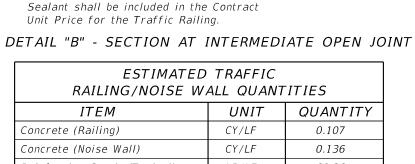
FY 2024-25 STANDARD PLANS

TRAFFIC RAILING/NOISE WALL (8'-0") - BRIDGE

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SHEET 3 of 5





INTERMEDIATE JOINT SEAL NOTES:

1. At Intermediate Open Joints,

bonding agent.

seal the lower 6" portion of

the open joint with Pre-cured

Silicone Sealant in accordance with Specification Section 932.

2. Apply sealant prior to any Class V

3. The cost of the Pre-cured Silicone

finish coating and remove all curing

compound and loose material from the surface prior to application of

RAILING/NOISE WALL QUANTITIES QUANTITY Concrete (Railing) 0.107 Concrete (Noise Wall) 0.136 LB/LF 69.36 Reinforcing Steel (Typical) Additional Reinf. @ Open Joint LB226.85

Paint Recessed Surfaces Black

Pre-cured Silicone

Sealant (4" wide)

SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

(The above quantities are based on the bridge mounted typical section, 2% deck cross slope and railing on low side of deck.)

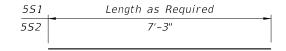




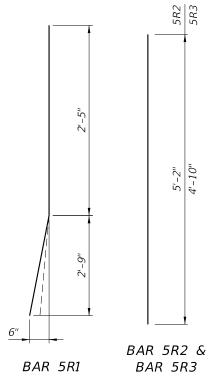


BILL OF REINFORCING STEEL MARK SIZE LENGTH R1 5 5'-2" R2 5 5'-2½" 5 R3 4'-10" 5 S 1 As Regd. 52 5 7'-3" V 5 6'-61/2"

BRIDGE		LOW G	UTTER	HIGH C	GUTTER
CROSS-SLOPE		ØA	ØB	ØA	ØB
E ED	0% to 2%	90°	90°	90°	90°
BRIDGE MOUNTEL	2% to 6%	93°	87°	87°	93°
B, MO	6% to 10%	96°	84°	84°	96°

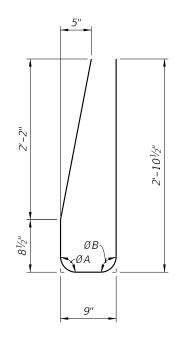


BARS 5S1 & 5S2



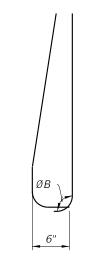
(Field Cut and Bend

for Railing End Transition)



STIRRUP BAR 5V

REINFORCING STEEL BENDING DIAGRAMS



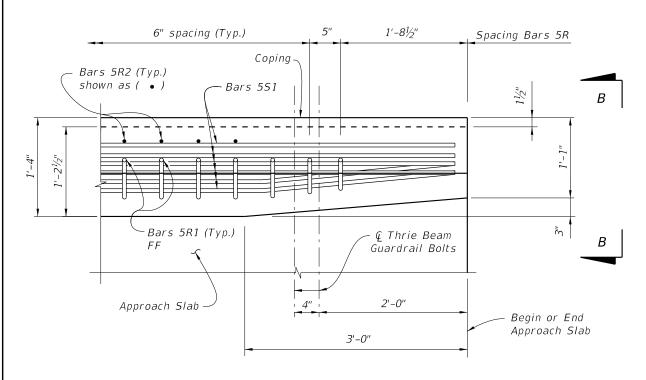
END STIRRUP BAR 5V To Be Field Cut (Railing End Transition)

REINFORCING STEEL NOTES:

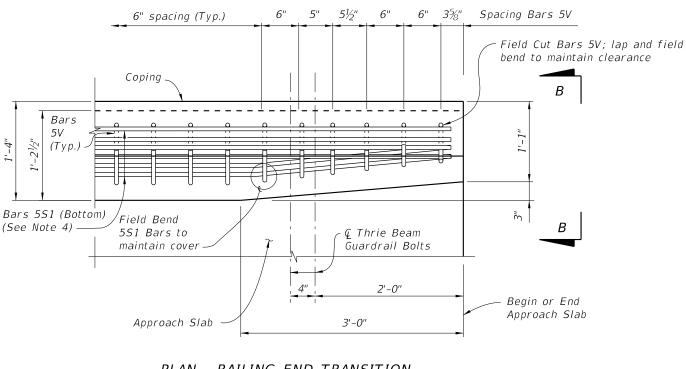
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.
- 4. Bars 5S1 may be continuous or spliced at the construction joints. Lap splices for Bars 5R2 and 5S1 shall be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

CROSS REFERENCE: For locations of Detail "B", see Sheet 1.

TRAFFIC RAILING/NOISE WALL (8'-0") - BRIDGE



PLAN - RAILING END TRANSITION (Showing Bars 5R, and Bars 5S1) (Bars 5V & Noise Wall Reinforcement not shown for Clarity)



PLAN - RAILING END TRANSITION (Showing Bars 5V and Bars 5S1) (Bars 5R not shown for Clarity)

= DETAIL "A" ===

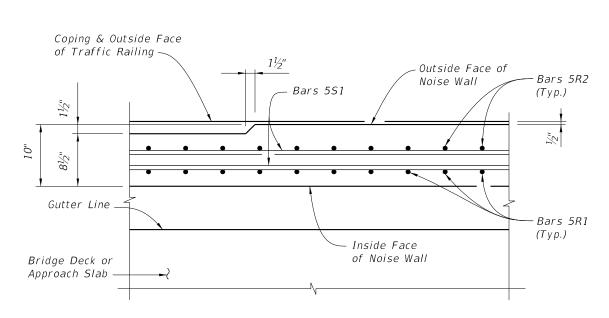
LAST O DESCRIPTION: REVISION 5 11/01/18

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FY 2024-25 STANDARD PLANS

DETAIL "A" NOTES:

- 1. Begin placing Railing Bars 5V at the railing end and proceed toward the guardrail (thrie beam) terminal connector to ensure placement of guardrail bolt holes. Pair Bars 5R with Bars 5V as shown. Clearance of Bars 5R & 5V to guardrail bolt holes shall be checked to prevent cutting of bars if holes are to be drilled. Shift bars locally where conflicts occur.
- 2. For Guardrail connection details see Index 536-001.
- . Omit Railing End Transition if a 36" Single-Slope Traffic Railing is used beyond the End Taper. See the Plan Sheets.
- 4. Field cut Bars 5R2 to maintain cover. Field cut Bars 5V and lap as necessary to maintain cover; field cut & bend Bars 5R1 front leg (more plumb) to maintain cover and tie to S1 Bars.



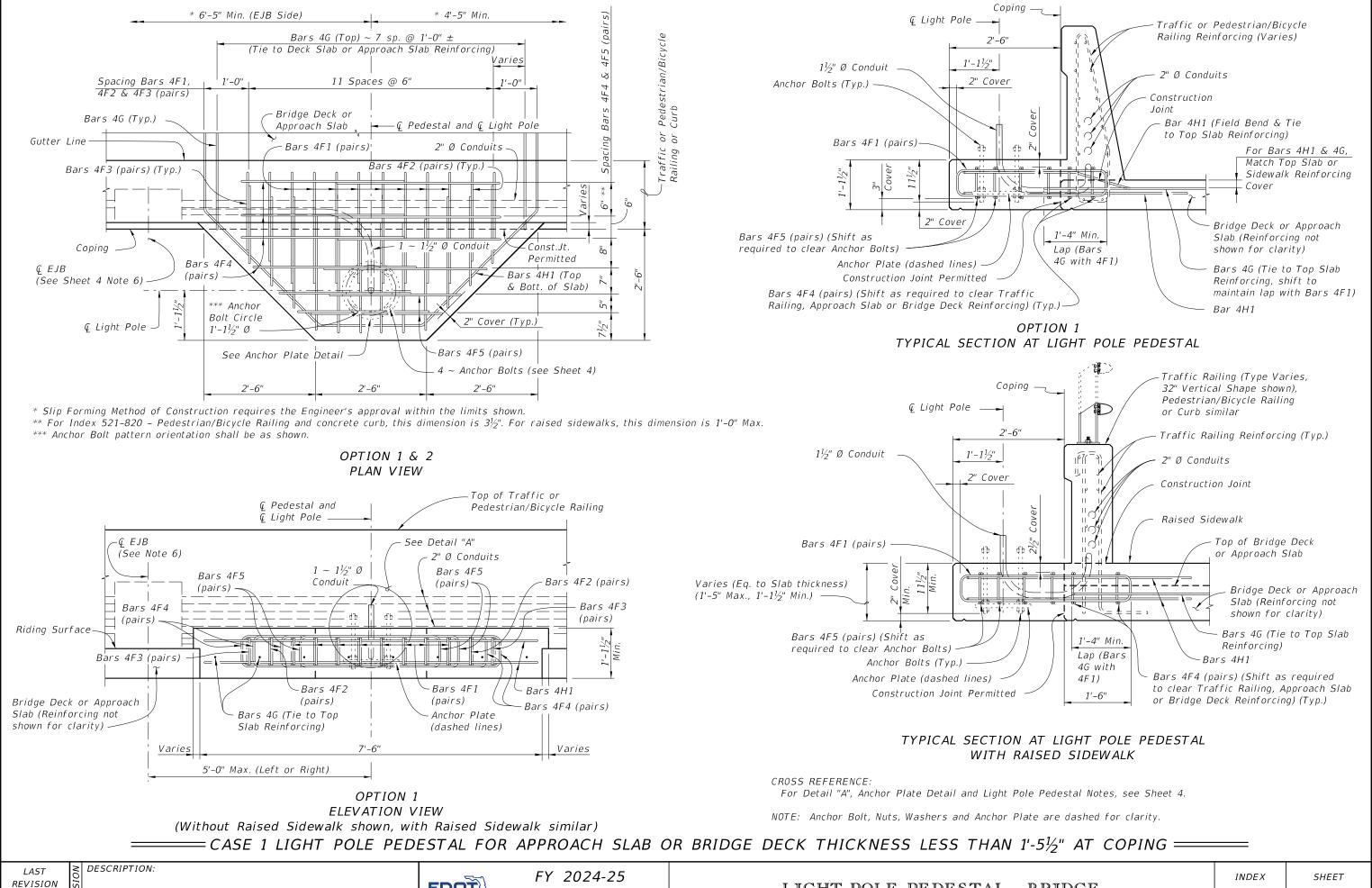
SECTION C-C THRU NOISE WALL END TAPER

CROSS REFERENCE:

For location of Detail "A" see Sheet 1. For location of Section C-C see Sheet 1. For View B-B see Sheet 3.

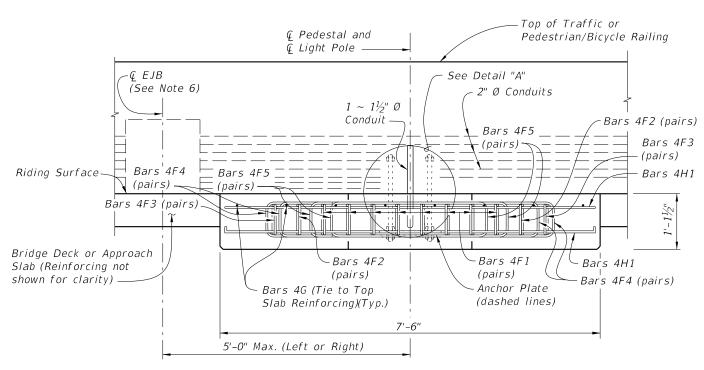
INDEX

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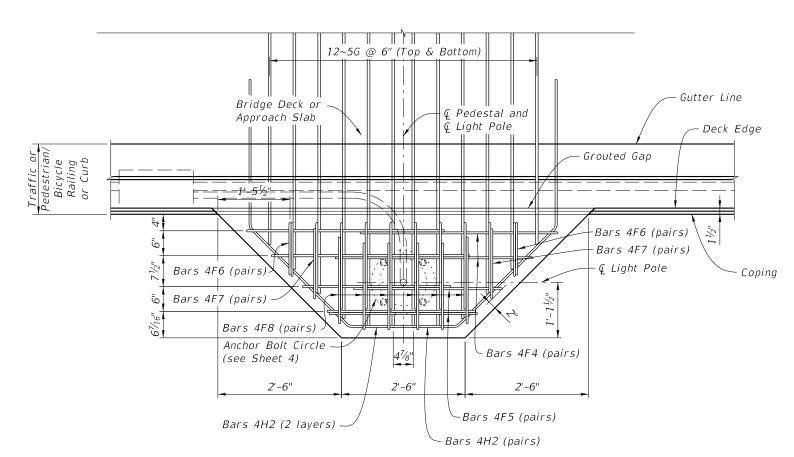


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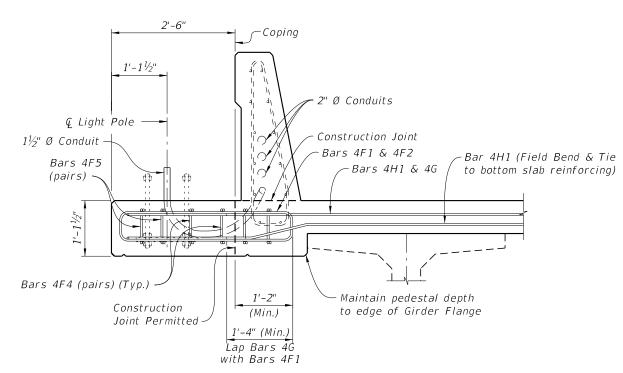
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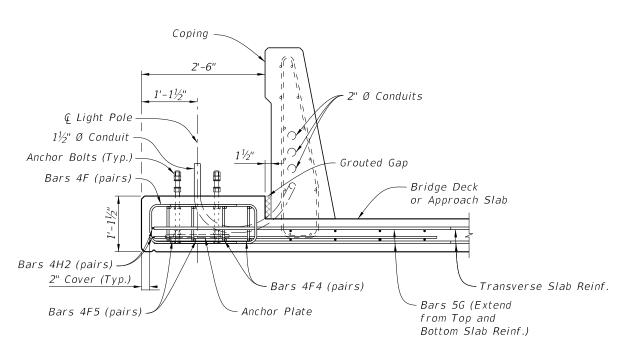
OPTION 2 - ELEVATION VIEW



OPTION 3 - PLAN VIEW WITH GAP BETWEEN BARRIER AND PEDESTAL TO ALLOW SLIP FORMING



OPTION 2 - TYPICAL SECTION AT LIGHT POLE PEDESTAL (Approach Slab Similar)



OPTION 3 - TYPICAL SECTION AT LIGHT POLE PEDESTAL WITH GAP BETWEEN BARRIER AND PEDESTAL TO ALLOW SLIP FORMING

CROSS REFERENCE:

For Detail "A", Anchor Plate Detail and Light Pole Pedestal Notes, see Sheet 4.

NOTE: Anchor Bolt, Nuts, Washers and Anchor Plate are dashed for clarity.

= CASE 1 LIGHT POLE PEDESTAL FOR APPROACH SLAB OR BRIDGE DECK LESS THAN 1'-5 $^1\!\!/_2$ " AT COPING =

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DESCRIPTION:

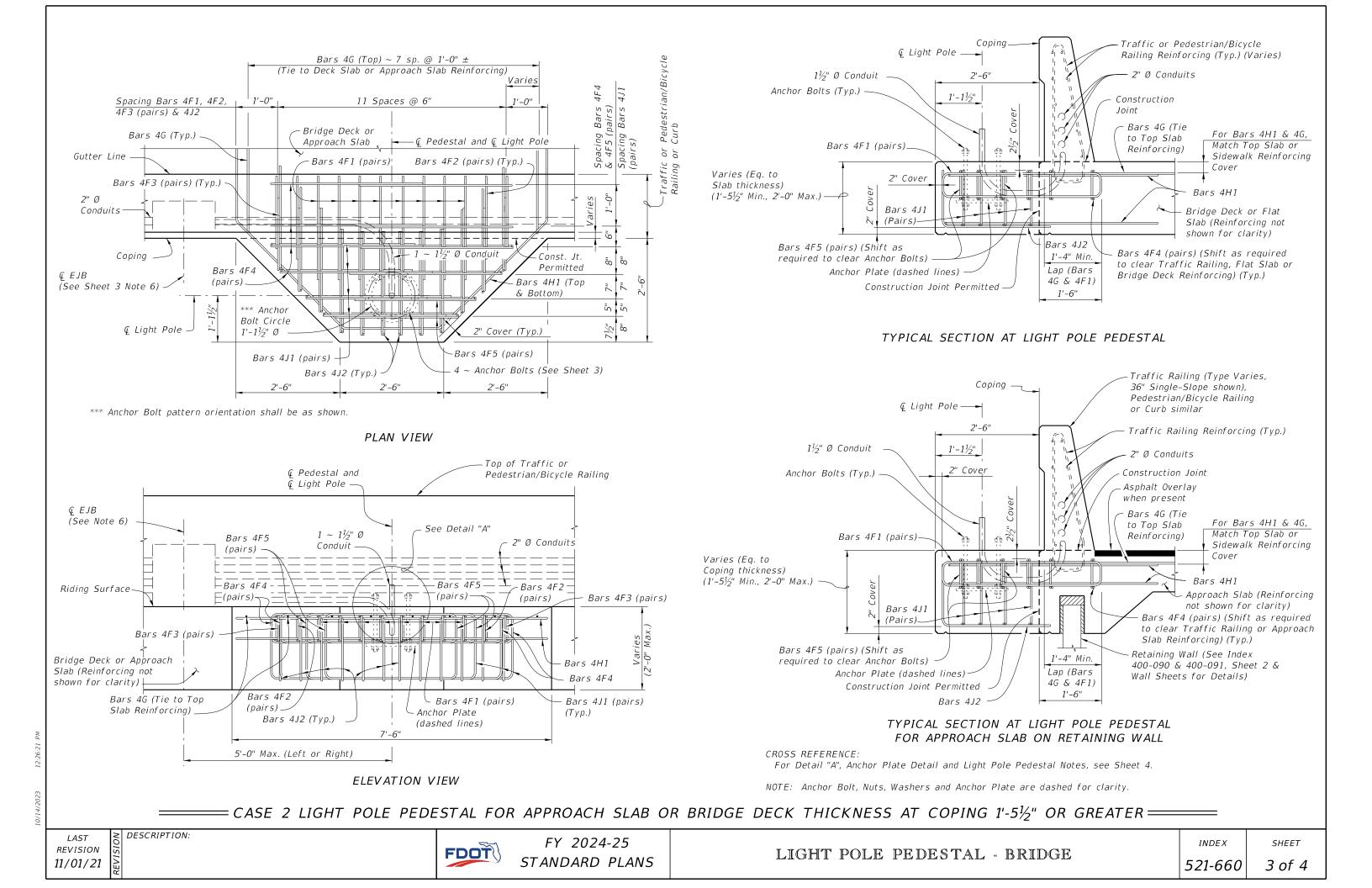
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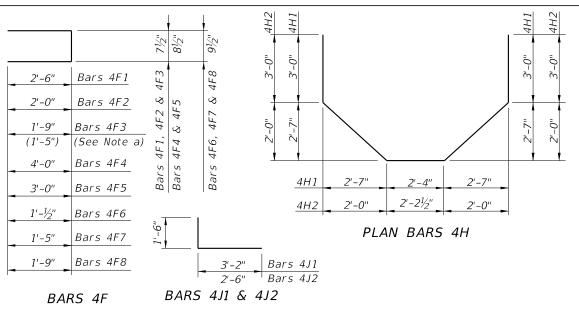
521-660



CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

REINFORCING STEEL NOTES:

- a. When Pedestal is attached to Pedestrian/Bicycle Railing Index 521-820 or an 8" wide concrete curb and the Bridge Deck or Approach Slab thickness is less than $1'-1\frac{1}{2}''$, Bars 4F3 shall have leg length and bar length shown in parentheses.
- b. The number of bars shown in parentheses is for Bars 4F4 when Pedestal is attached to Pedestrian/Bicycle Railing - Index 521-820 or an 8" wide concrete curb, and the Bridge Deck or Approach Slab thickness is less than $1'-1\frac{1}{2}''$.
- c. Lap Splices for Bars 4F1, 4F2 & 4F3 shall be a minimum of 1'-4". Lap Splices for Bars 4F4 & 4F5 shall be minimum of 1'-8".
- d. Bars 4J1 and 4J2 are not required when Pedestal thickness is less than $1'-5\frac{1}{2}$ ". Field trim height of bars to maintain cover when Pedestal thickness is less than 2'-0". Field trim length of Bars 4J2 on Retaining Wall Coping to maintain cover.
- e. All bar dimensions in the bending diagrams are out to out.



BILL OF REINFORCING STEEL				
MARK	SIZE	NO. REQD.	LENGTH	NOTES
F 1	4	16	5'-8"	С
F2	4	4	4'-8"	С
F3	4	4	4'-2'' (3'-6")	а, с
F 4	4	4 8 (6) [4 for Option 3]		b, c
F5	4	4	6'-9"	С
F6	4	4	2'-11"	-
F7	4	4	3'-8"	-
F8	4	12	4'-4"	-
G	4 [5 for Option 3]	8 [24 for Option 3]	6'-0"	-
H1	4	2	15'-8"	-
H2	4	2	13'-10"	-
J 1	4	8	4'-8"	d
J2	4	12	4'-0"	d

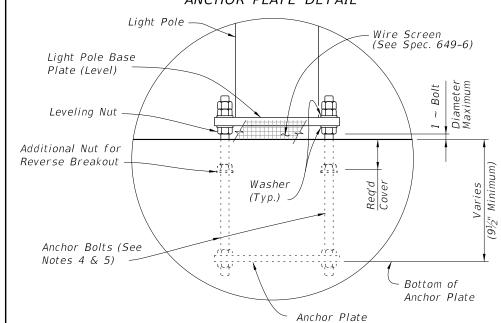
() See Reinforcing Steel Note a & b.

1'-31/5' 111/5"

 $4 \sim (Bolt \ Dia. + \frac{1}{16}") \ \emptyset$ Holes equally spaced

 $1'-1\frac{1}{5}''$ Ø bolt hole circle

ANCHOR PLATE DETAIL



DETAIL "A"

LIGHT POLE PEDESTAL NOTES

1. Concrete and Reinforcing Steel required for the construction of the Pedestal shall meet the same requirements as the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.

6'-0"

BAR 5G

2. Light Pole Pedestal may be used with the following:

Index 521-422 - Traffic Railing (42" Vertical Shape),

Index 521-423 - Traffic Railing (32" Vertical Shape),

Index 521-427 - Traffic Railing (36" Single-Slope), Index 521-428 - Traffic Railing (42" Single-Slope),

Index 521-820 - Pedestrian/Bicycle Railing,

Index 515-021 - Pedestrian/Bicycle Bullet Railing for

Traffic Railing or

Index 515-509 - Traffic Railing /Noise Wall - Bridge.

- 3. Unless otherwise noted, Traffic Railing (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other Traffic Railings or Pedestrian/Bicycle Railing are similar.
 - TABLE 1 DESIGN LIMITATIONS FOR ANCHOR BOLTS (1" Dia.) BRIDGE DECK HEIGHT (Ft.)* ARM WIND SPEED LENGTH DESIGN MOUNTING HEIGHT (MPH) (Ft.) 40 Ft. 45 Ft. 50 Ft. 130 ≤ 15 75 75 75 150 ≤ 15 75 75 75 8 & 10 75 75 45** 170
 - * Above natural ground or MLW.
 - ** Use $1\frac{1}{4}$ " diameter Anchor Bolt for Bridge Deck Height greater than shown, in Table 1, up to 75'.

25**

4. ANCHOR BOLTS:

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole configurations shown on Index 715-002.

Anchor Bolt Diameter: See Table 1 Anchor Bolts: ASTM F1554 Grade 55.

Nuts: ASTM A563 Grade A, Heavy-Hex.

Washers: ASTM F436 Type 1.

Anchor Plate: ASTM A709 (Grade 36) or ASTM A36.

Coating: Galvanize all Nuts, Bolts Washers, in accordance with ASTM F2329. Galvanize plates in accordance with ASTM A123.

The Contractor is responsible for ensuring the anchor bolt configuration is compatible with the light pole base plate. Submit modifications of the anchor bolt design to the Engineer for approval.

- 5. Install Anchor Bolts plumb.
- 6. For Conduit, Embedded Junction Boxes (EJB), Expansion/Deflection Fitting and adjacent Reinforcing Steel Details, see Utility Conduit Detail Sheets and Index 630-010.
- 7. PAYMENT: The cost of Wire Screen, Anchor Bolts, Nuts, Washers and Anchor Plates shall be included in the Bid Price for Light Poles. The cost of all Labor, Concrete and Reinforcing Steel required for the Construction of the Pedestals, and Miscellaneous Hardware required for the completion of the Electrical System, shall be included in the Bid Price for the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.

ESTIMATED LIGHT POLE PEDESTAL QUANTITIES PER LIGHT POLE PEDESTAL			
ITEM UNIT QUANTITY			
Concrete Per Pedestal Thickness	CY/In.	0.040	
Reinforcing Steel	LB	195 (182)	

(The Reinforcing Steel quantity shown in parenthesis is for a Pedestal attached to Pedestrian/Bicycle Railing - Index 521-820 with Bridge Deck or Approach Slab thinner than 1'-11/2". Add 59 Lbs. for Bars 4J1 & 4J2 when Pedestal Thickness is 1'-51/2" or greater)

For location of Detail "A" see Sheets 1,2 and 3. REVISION 11/01/21

DESCRIPTION:

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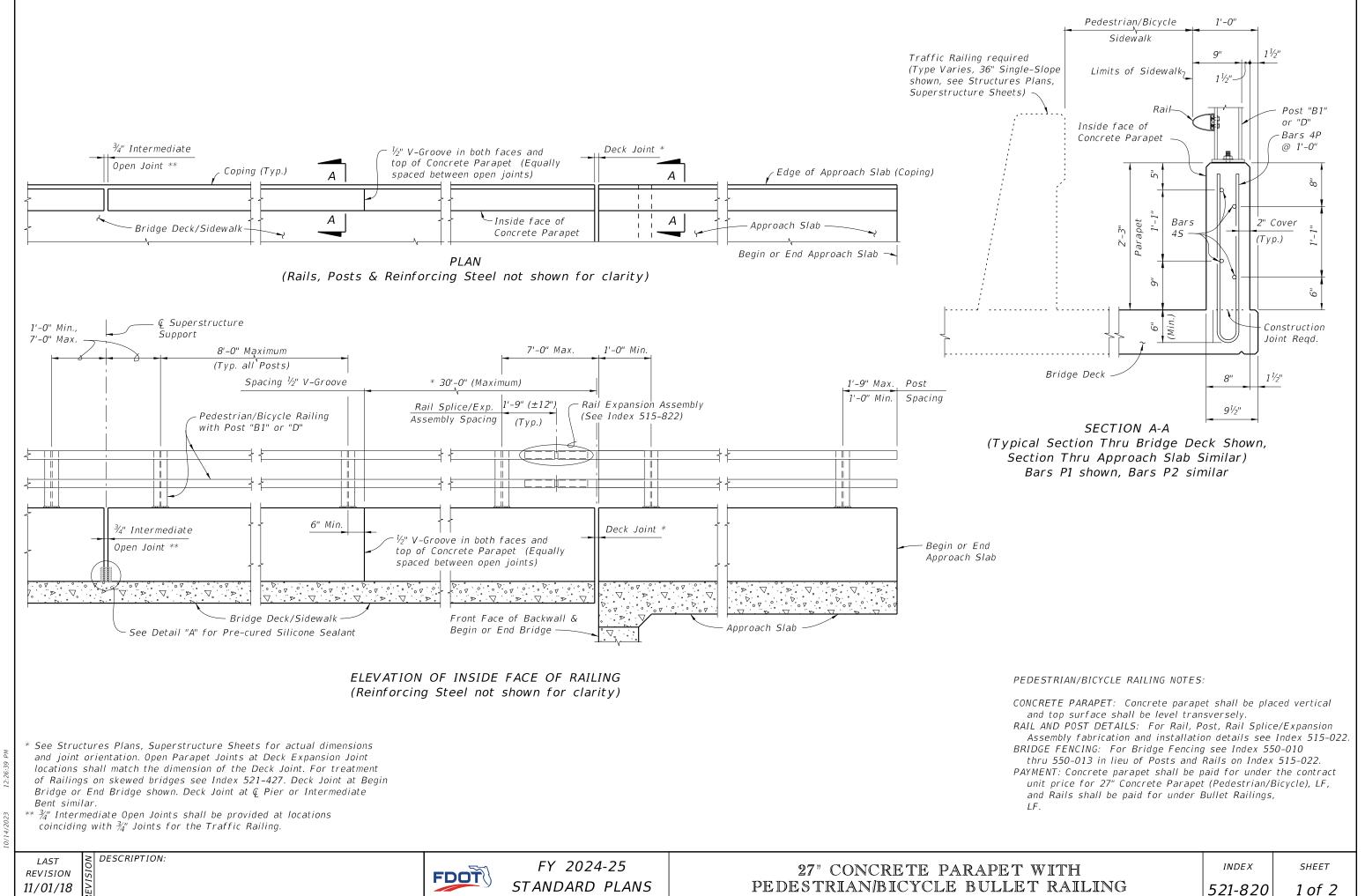
170 | 12 & 15

LIGHT POLE PEDESTAL - BRIDGE

INDEX 521-660

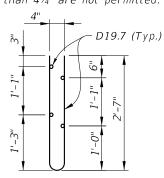
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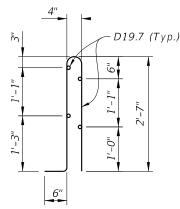
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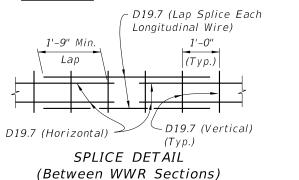
ALTERNATE REINFORCING (WELDED WIRE REINF.) DETAILS

NOTE: Place wire panels to minimize the end overhang. End Overhangs greater than $4\frac{3}{4}$ " are not permitted.





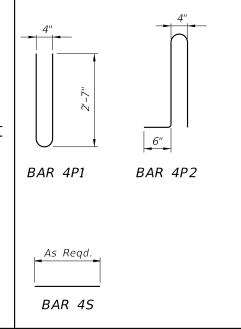
:WELDED WIRE REINFORCEMENT (WWR) ____



CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

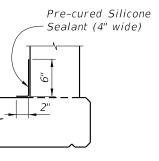
BILL OF REINFORCING STEEL

MARK	SIZE	LENGTH
P 1	4	5'-6"
P2	4	6'-0"
S	4	As Read.



REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the parapet on a retaining wall shall be the same as detailed above for a 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Bars 4P2 may be used in lieu of Bars 4P1.
- 6. At the option of the Contractor deformed WWR may be used in lieu of all Bars 4P or 4P2 and 4S.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTE:

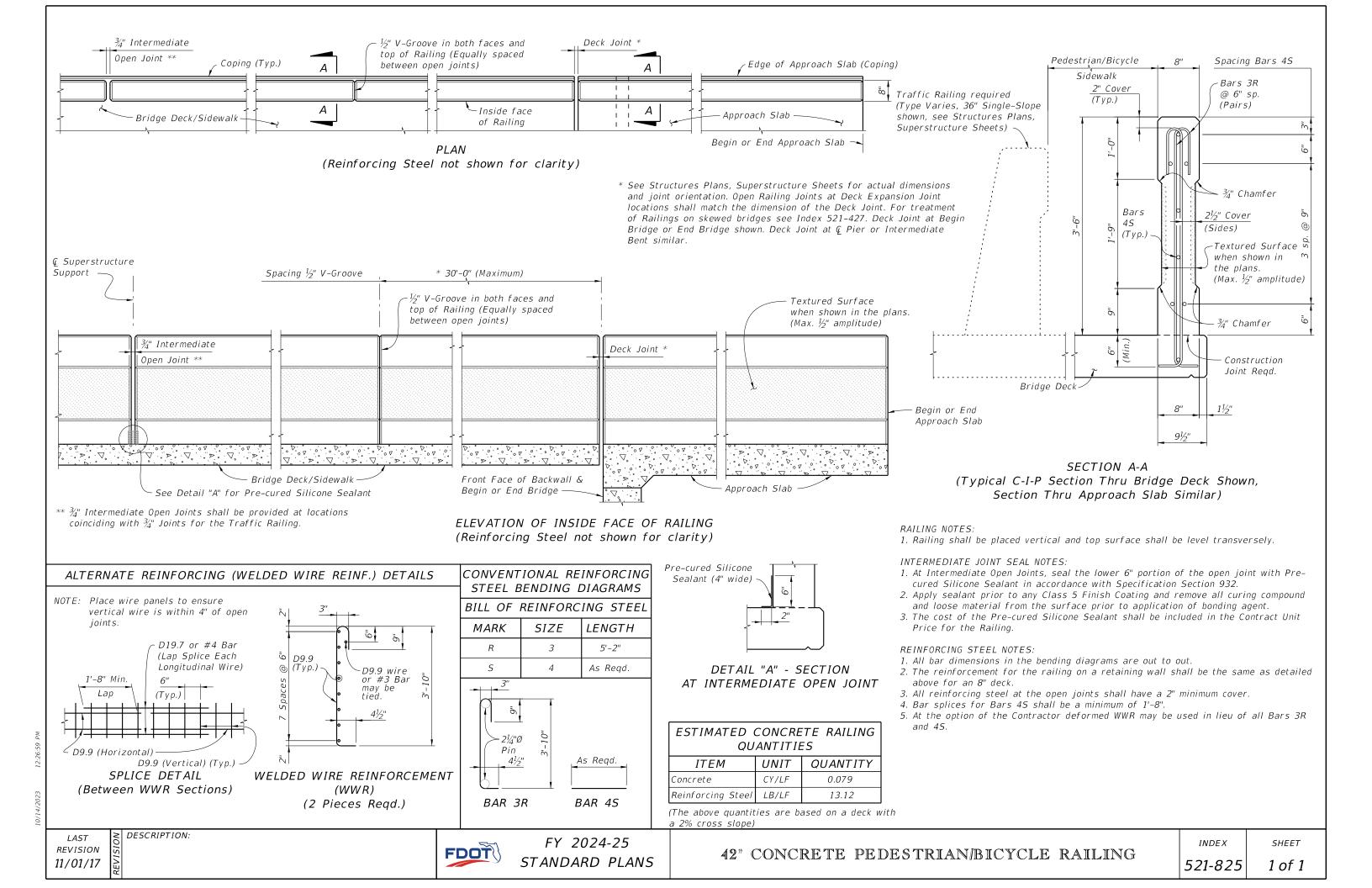
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant meeting the requirements of Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Concrete Parapet.

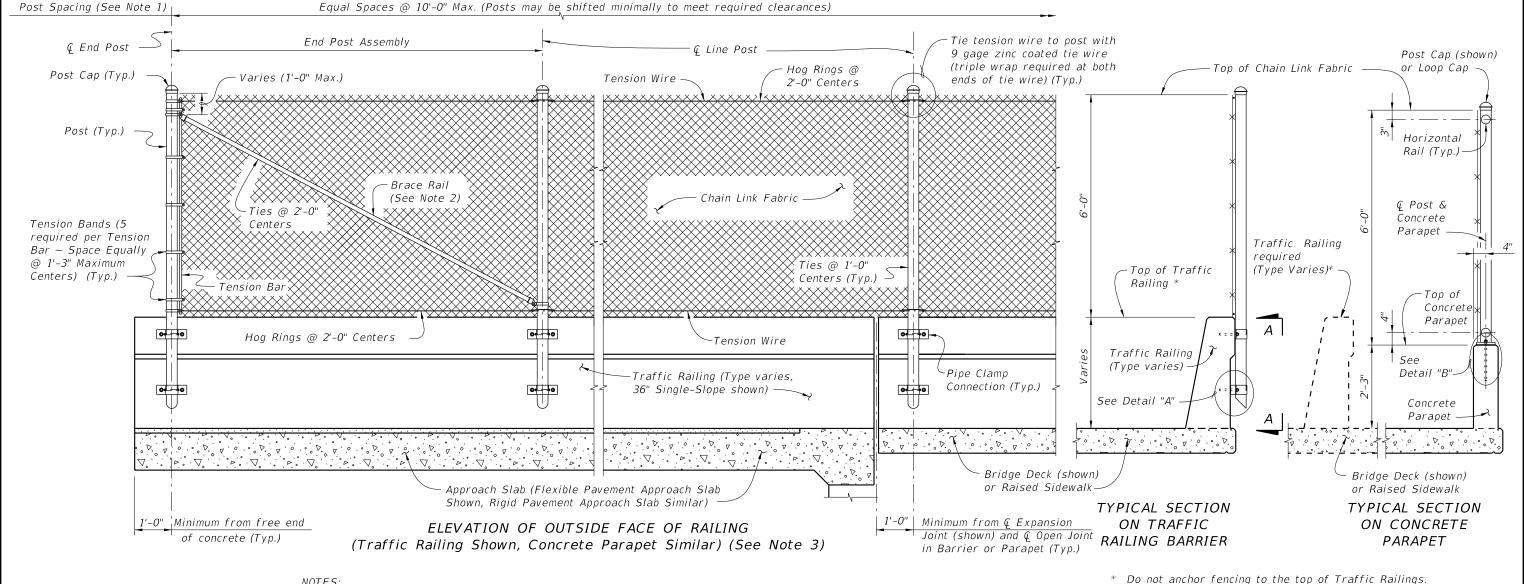
ESTIMATED CONCRETE PARAPET QUANTITIES			
ITEM UNIT QUANTITY			
Concrete	CY/LF	0.056	
Reinforcing Steel (P1 & S)	LB/FT	6.35	
Reinforcing Steel (P2 & S)	LB/FT	6.68	

(The above quantities are based on a deck with a 2% cross slope)

DESCRIPTION:







- 1. A Pull Post Assembly is required at maximum intervals of 500'-0". See Sheet 3.
- 2. Brace rails are only required for vertical fence installations on Traffic Railing.
- 3. Provide horizontal rails for vertical fence installations on Concrete Parapets in lieu of tension wire. Locate horizontal rails as shown in the Typical Section for Concrete Parapets at right.

FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

CONCRETE PARAPET DETAILS:

DESCRIPTION:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace rails and bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, post and loop caps, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, bearing pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

For Pull Post Assembly Detail for Traffic Railings see Sheet 3.

For Pull Post Assembly Detail for Concrete Parapets and Detail "B" see Sheet 4.

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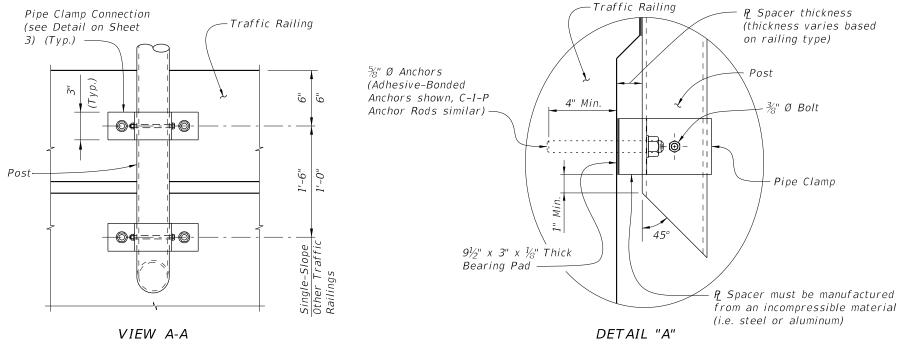
BRIDGE FENCING (VERTICAL)

INDEX 550-010

SHEET 1 of 4

TABLE OF CHAIN LINK FENCE COMPONENTS				
COMPONENT ASTM DESIGNATION			COMPONENT INFORMATION	
	Posts	F1083	Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade	
ets	Chain Link Fabric (2" mesh with twisted top and knuckled bottom selvage)	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating	
		A491	Aluminum Coated Steel - 9 gage (coated wire diameter)	
lings Parap		F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b	
Traffic Railings and Concrete Parapets	Tie Wires	F626	Zinc Coated Steel Wire - 9 gage	
Traff Conc	Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)	
and	Tension Bars	F626	$\frac{3}{16}$ " (Min. thickness) x $\frac{3}{4}$ " (Min. width) x 5'-10" (Min. height) Steel Bars	
	Tension Bands	F626	14 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands	
	Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)	
	Horizontal Rails	F1083	Galvanized Steel Pipe – $2\frac{1}{2}$ " NPS, Schedule 40 Regular Grade	
e S:	Expansion Rails	F1083	Galvanized Steel Pipe - 2" NPS, Schedule 40 Regular Grade	
Concrete Parapets	Bolts	A307	$\frac{1}{4}$ " Ø x $4\frac{1}{4}$ " Hex Head Bolts for Expansion Rail Connections	
CC	Nuts	A563	Hex Nuts for Expansion Rail Connections	
	Washers	F436	Flat Washers for Expansion Rail Connections	
gs	Tension Wire A824	4024 C 4017	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating	
ailin		A824 & A817	Type I (Aluminum Coated Steel Wire) - 7 gage	
Traffic Railings	Hog Rings	F626	Zinc Coated Steel Wire - 12 gage	
Traf	Brace Rails	F1083	Galvanized Steel Pipe – $1\frac{1}{4}$ " NPS, Schedule 40 Regular Grade	

TABLE OF POST ATTACHMENT COMPONENTS				
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe	Clamps	A36 or A709 Grade 36	1/4" Steel P	
Base	Plates	A36 or A709 Grade 36	¾" Steel Æ	
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $\frac{3}{4}$ " Ø	
Space	ers	-	Plate thickness varies based on traffic railing type (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Pipe Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{1}{2}$ " Ø x 6" (no spacer) or $\frac{1}{2}$ " Ø x (6" + spacer thickness)	
lase Plate	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods \sim 7_8 " Ø x $14\frac{1}{2}$ "	
Base Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections	
Washers		F436	Flat Washers for Pipe Clamp and Base Plate Connections	
Bearing Pads (Plain Neoprene)		-	In accordance with Specification Section 932 for Ancillary Structures	



POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

WELDING:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

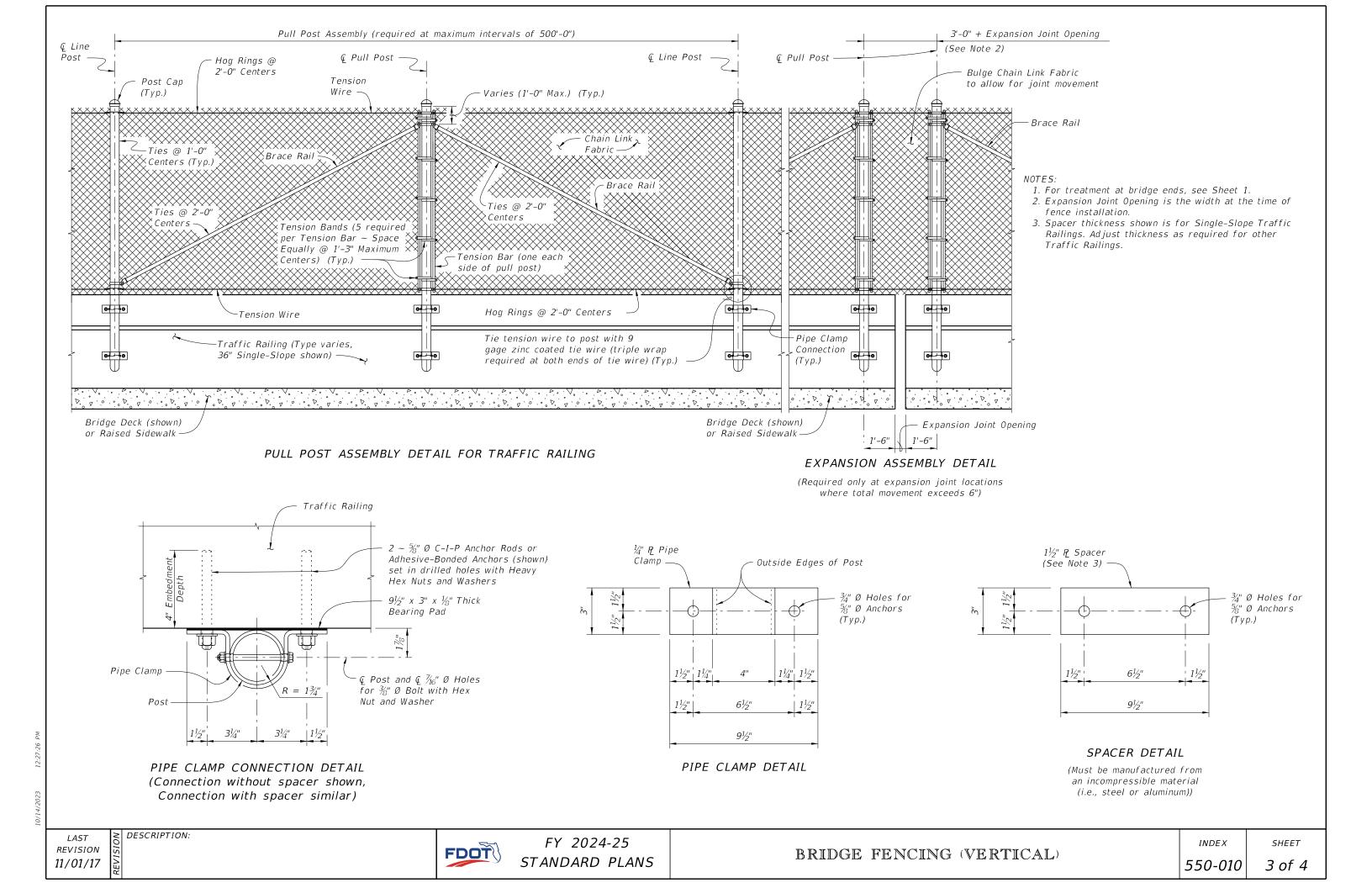
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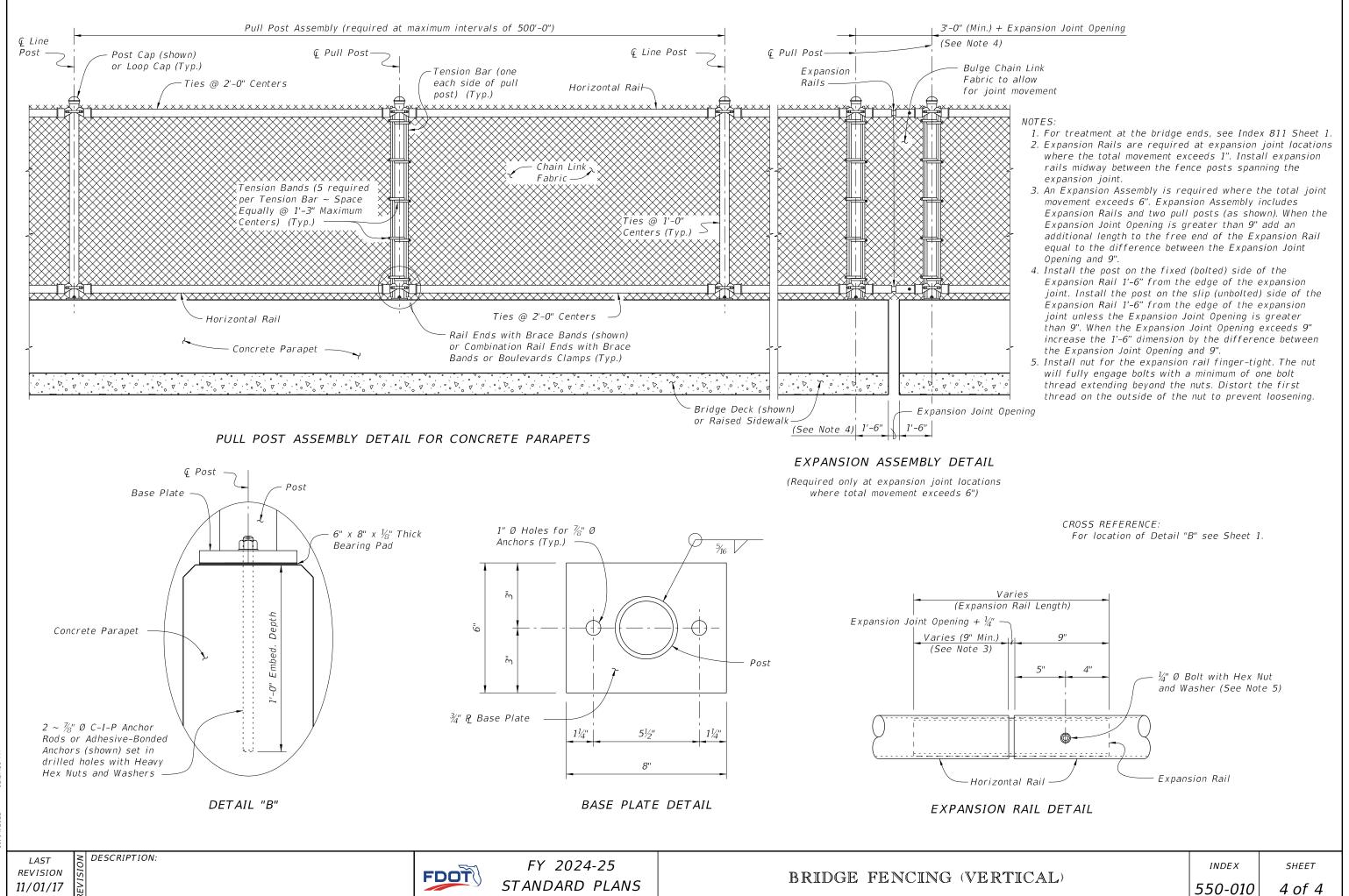
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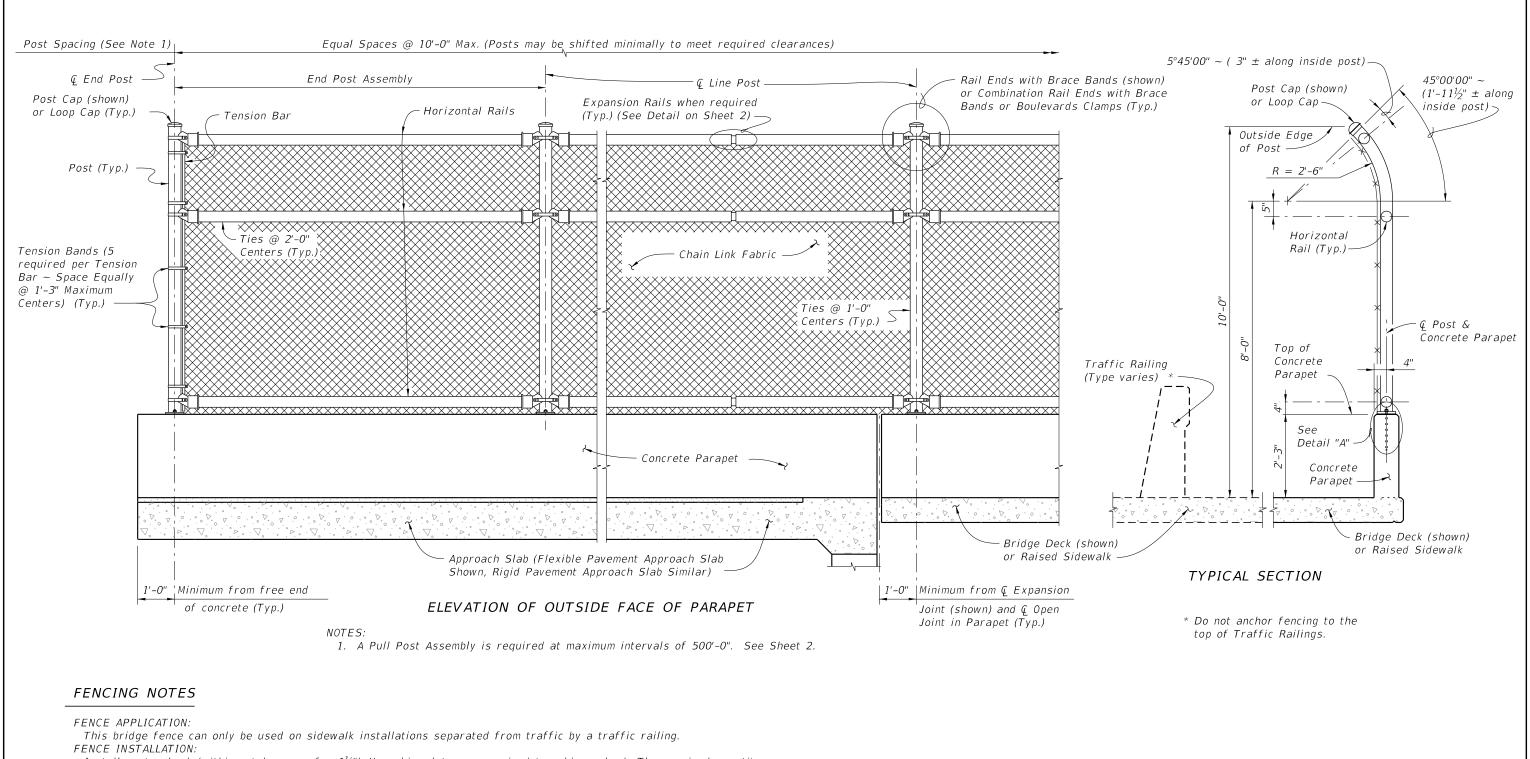
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Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable.

CONCRETE PARAPET DETAILS:

See Index 521-820 - Pedestrian/Bicycle Bullet Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

DESCRIPTION:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, ties, tension bars and bands, post and loop caps, base plates, anchor rods, bolts, nuts, washers, shim plates, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components and Pull Post Assembly Detail see Sheet 2. For Table of Post Attachment Components and Detail "A" see Sheet 3.

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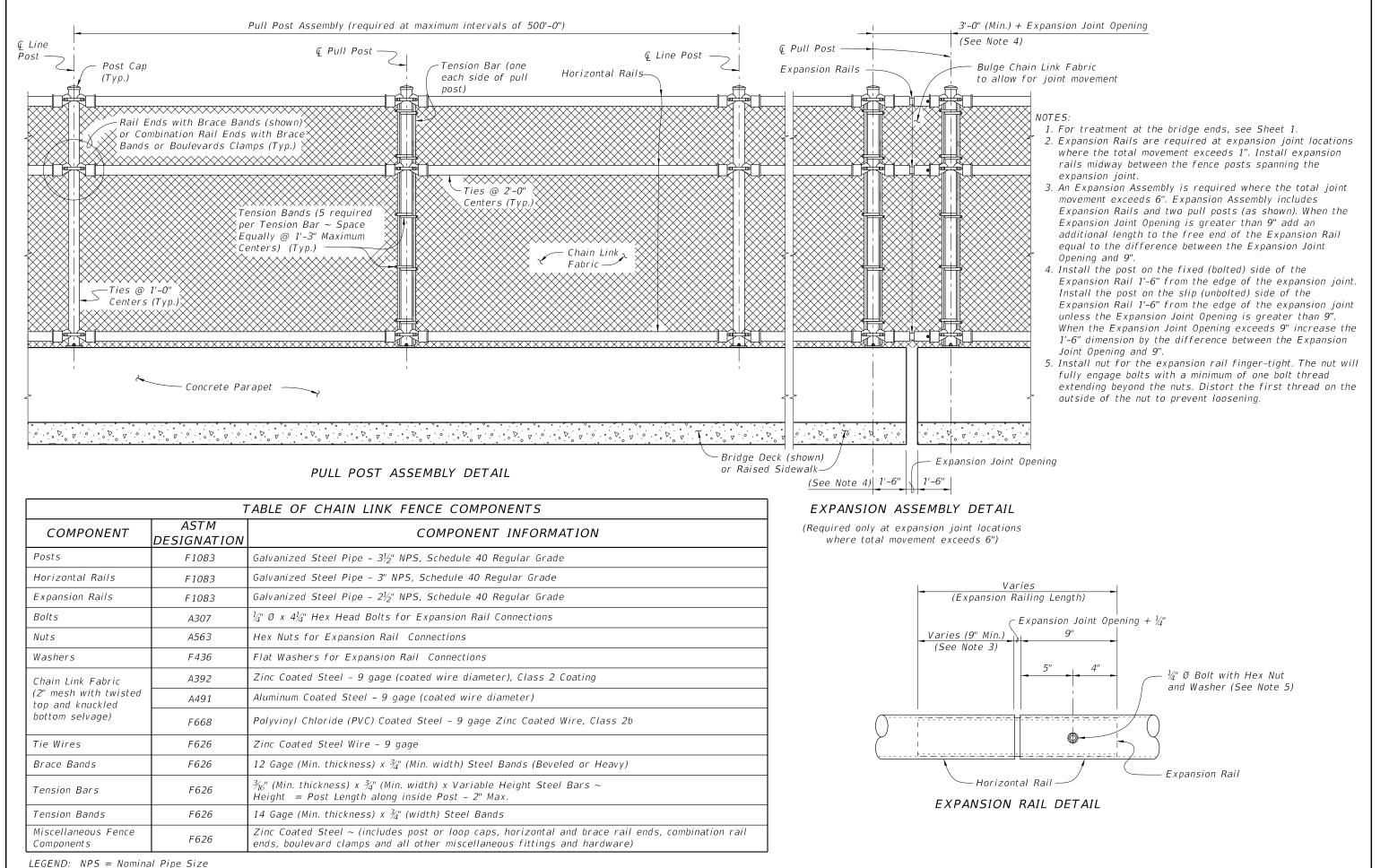
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BRIDGE FENCING ON PARAPET (CURVED TOP)

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DESCRIPTION:

FDOT

TABLE OF POST ATTACHMENT COMPONENTS					
COMPONENT ASTM DESIGNATION		COMPONENT INFORMATION			
Base Plates	A36 or A709 Grade 36	¾" Steel P			
A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5		Plate thicknesses as required, Holes in shim plates will be $\frac{3}{4}$ " Ø			
Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "			
C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "			
Nuts A563		Hex Nuts for Base Plate Connections			
Washers F436		Flat Washers for Base Plate Connections			
Bearing Pads (Plain) -		In accordance with Specification Section 932 for ancillary structures			

POST ATTACHMENT NOTES

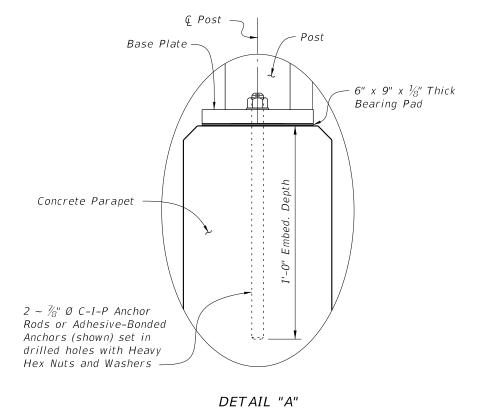
ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates and Base Plates) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication. ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.



1" Ø Holes for ½" Ø Anchors (Typ.) Post ¾" P_Base Plate $6\frac{1}{2}$ " 9"

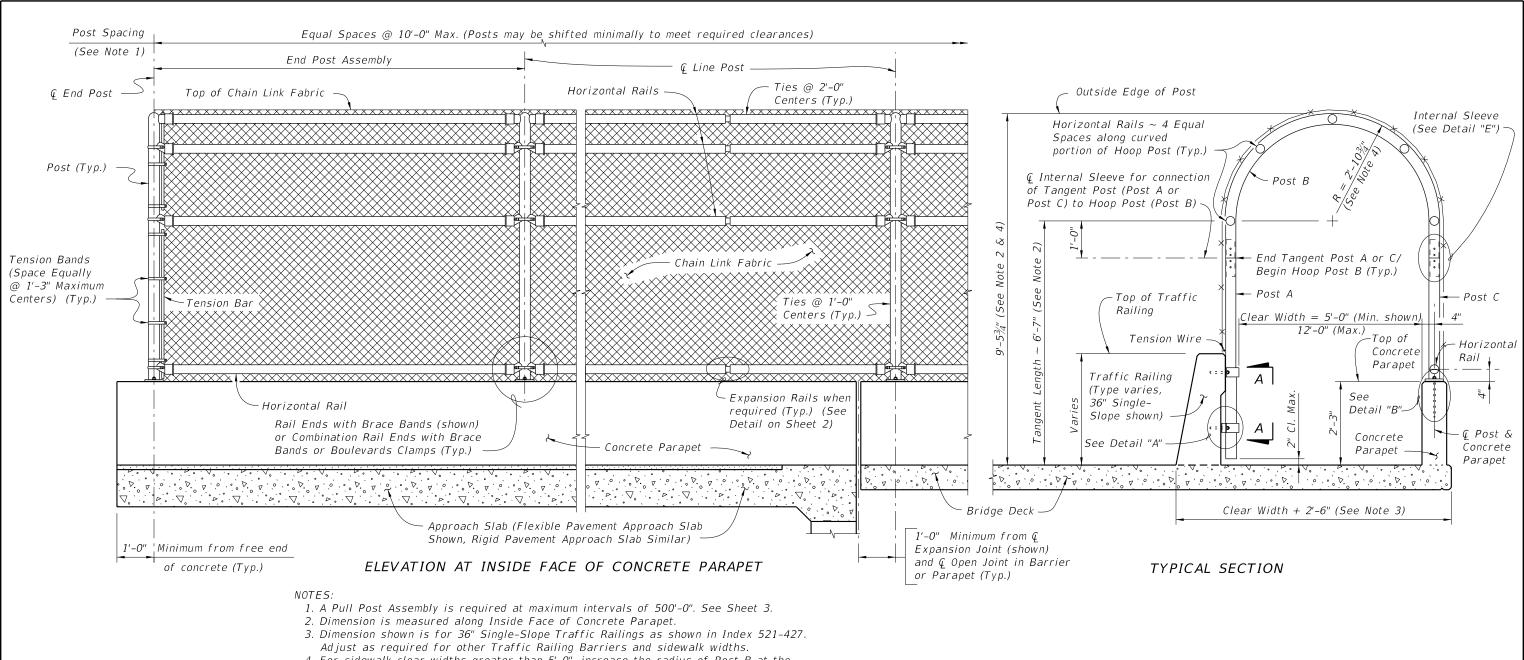
BASE PLATE DETAIL

CROSS REFERENCE: For location of Detail "A" see Sheet 1.

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DESCRIPTION:





4. For sidewalk clear widths greater than 5'-0", increase the radius of Post B at the rate of 6" for every one foot increase in sidewalk width.

FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as

TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

CONCRETE PARAPET DETAILS:

DESCRIPTION:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components and Table of Post Attachment Components see Sheet 2. For Pull Post Assembly Detail, View A-A and Detail "A" see Sheet 3.

For Detail "B" and "E" see Sheet 4.

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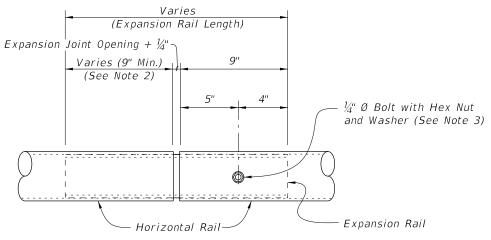
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TABLE OF CHAIN LINK FENCE COMPONENTS			
COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION	
Posts	F1083	Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade	
Horizontal Rails and Internal Sleeves	F1083	Galvanized Steel Pipe - $2\frac{1}{2}$ " NPS, Schedule 40 Regular Grade	
Expansion Rails	F1083	Galvanized Steel Pipe - 2" NPS, Schedule 40 Regular Grade	
Chain Link Fabric (2" mesh with knuckled bottom selvages)	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating	
	A491	Aluminum Coated Steel - 9 gage (coated wire diameter)	
	F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b Zinc Coated Wire	
	A824 & A817	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating	
Tension Wire		Type I (Aluminum Coated Steel Wire) - 7 gage	
Tie Wires	F626	Zinc Coated Steel Wire - 9 gage	
Hog Rings	F626	Zinc Coated Steel Wire - 12 gage	
Brace Bands	F626	12 gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)	
Tension Bars	ension Bars $\frac{3_{16}''}{16}$ (Min. thickness) x $\frac{3_4''}{4}$ (Min. width) x Variable Height Steel Bars ~ Height = Tangent or Hoop Length - Barrier or Parapet Height - 2" max.		
Tension Bands	F626	14 gage (Min. thickness) x $rac{3}{4}$ " (Min. width) Steel Bands	
Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes horizontal rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings and hardware)	
Bolts	A307	$\frac{3}{8}$ " Ø x $4\frac{1}{4}$ " Hex Head Bolts for Internal Sleeve connections $\frac{1}{4}$ " Ø x $4\frac{1}{4}$ " Hex Head Bolts for Expansion Rail connections	
Nuts	A563	Hex Nuts for Internal Sleeve and Expansion Rail connections	
Washers	F436	Flat Washers for Internal Sleeve and Expansion Rail connections	



NOTES:

EXPANSION RAIL DETAIL

- 1. Expansion Rails are required at expansion joint locations where the total movement exceeds 1". Install expansion rails midway between the fence posts spanning the expansion joint.
- 2. An Expansion Assembly is required where the total joint movement exceeds 6". Expansion Assembly includes Expansion Rails and two pull posts (see Sheet 3). When the Expansion Joint Opening is greater than 9" add an additional length to the free end of the Expansion Rail equal to the difference between the Expansion Joint Opening and 9".
- 3. Install nut for the expansion rail finger-tight. The nut will fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening.

TABLE OF POST ATTACHMENT COMPONENTS				
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe Clamps		A36 or A709 Grade 36	¼" Steel ዊ	
Base Plates		A36 or A709 Grade 36	¾" Steel PL	
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $^3\!4''$ Ø	
Spacers		-	Plate thickness varies based on Traffic Railing type. (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Pipe Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods \sim $\%$ " Ø x 6" (no spacer) or 5% " Ø x (6" + spacer thickness)	
Base Plate Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods \sim $\%$ 0 x 14 $\%$	
Base Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections	
Washers		F 436	Flat Washers for Pipe Clamp and Base Plate Connections	
Bearing Pads (Plain)		-	In accordance with Specification Section 932 for Ancillary Structures	

POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

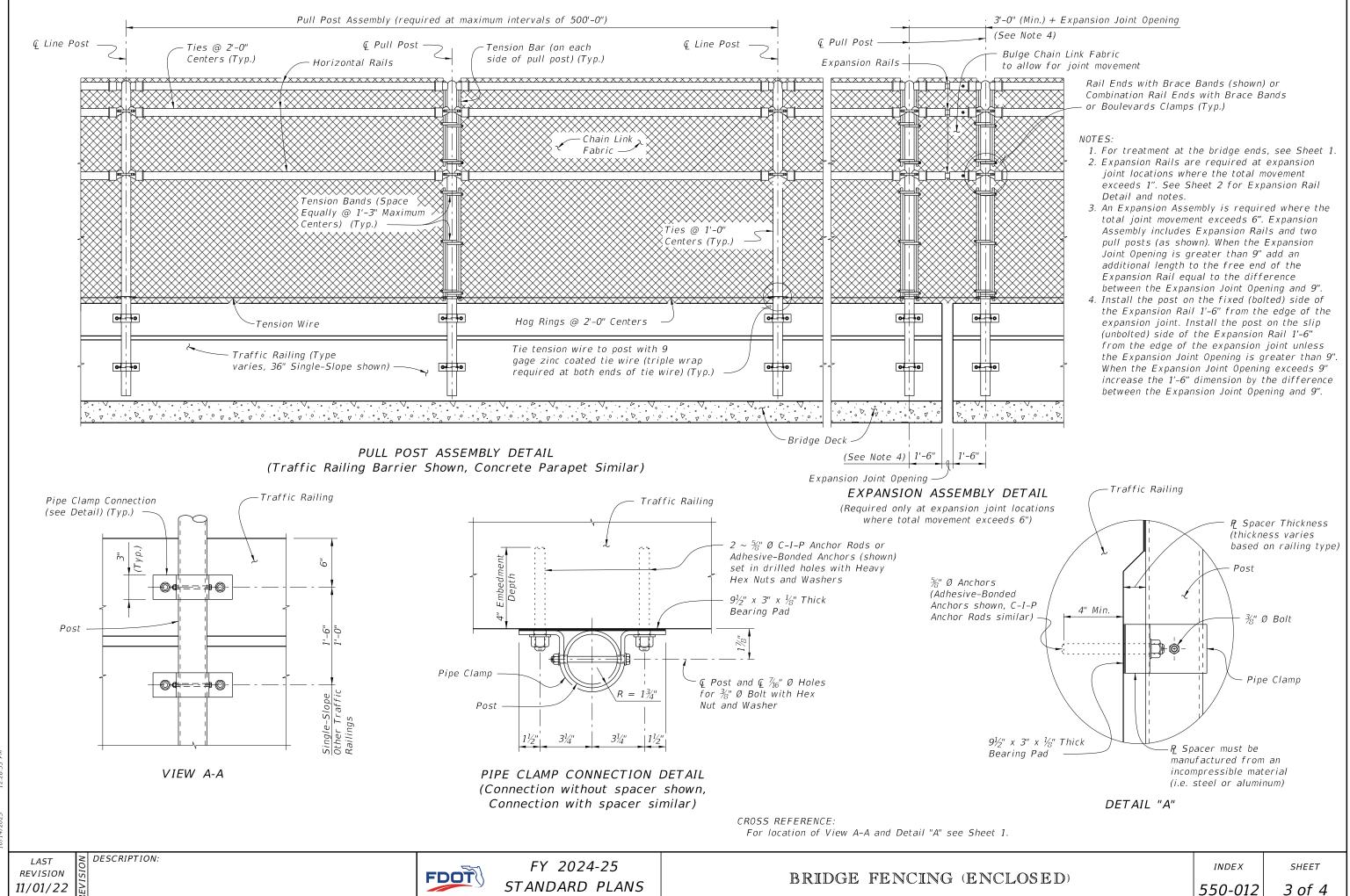
WELDING:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

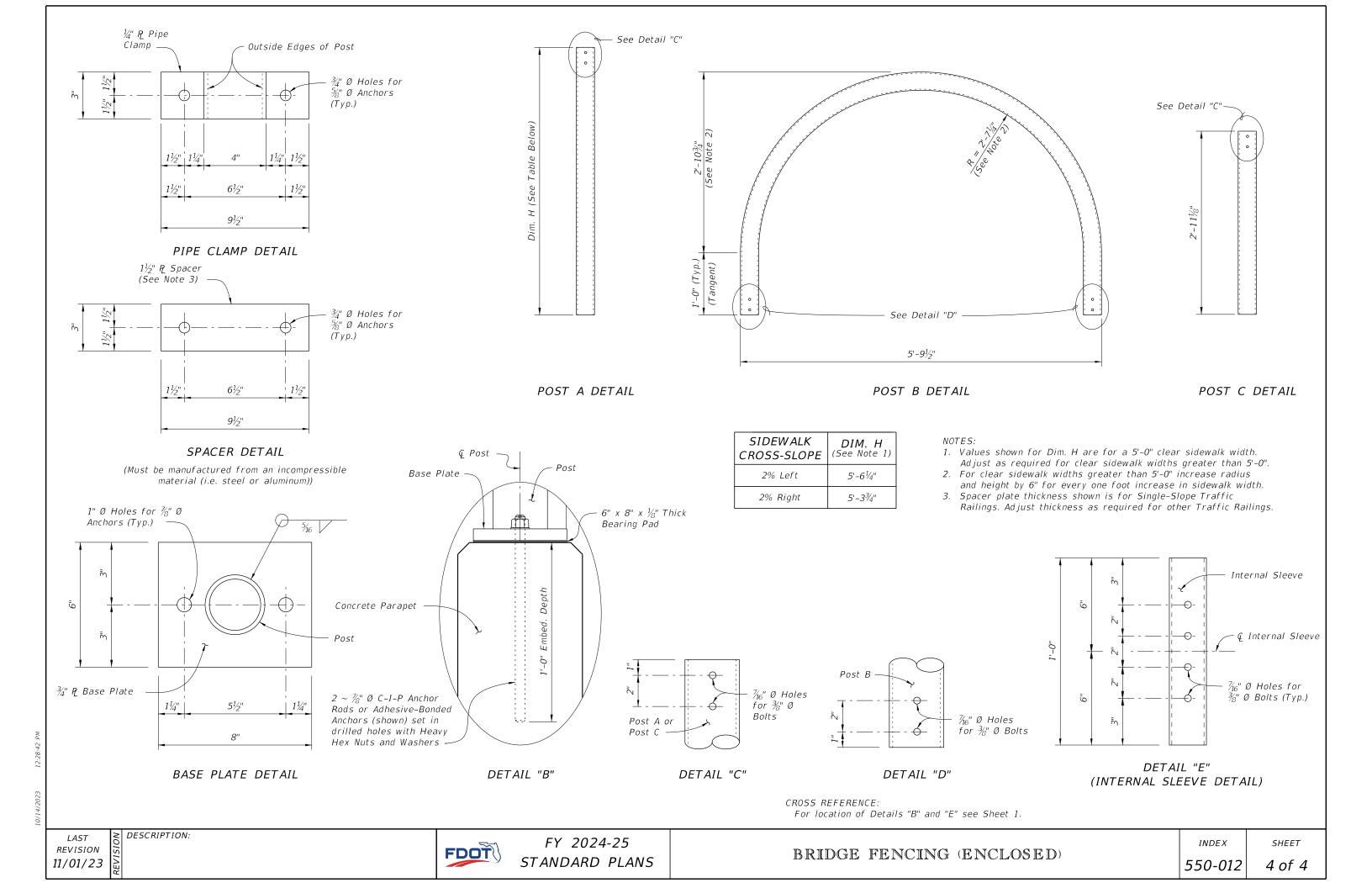
REVISION 11/01/17

DESCRIPTION:



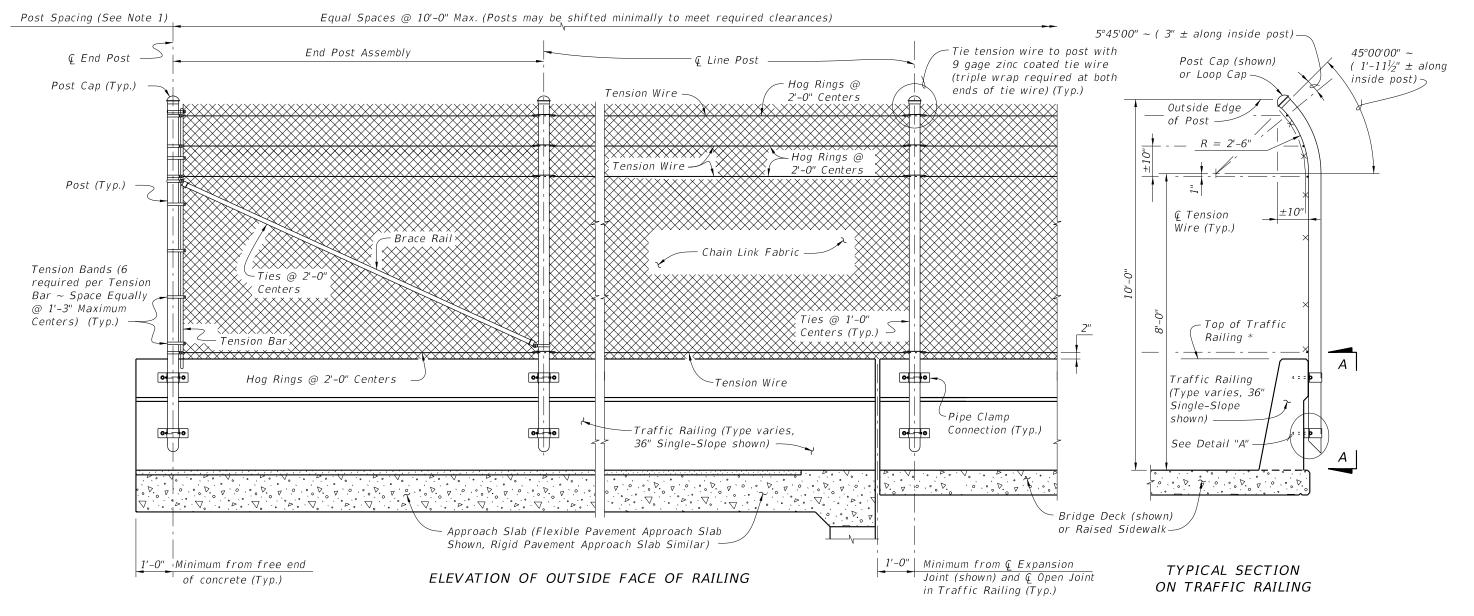


10/14/3033









NOTES:

1. A Pull Post Assembly is required at maximum intervals of 500'-0". See Sheet 3.

* Do not anchor Fencing to the top of Traffic Railings.

FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes all materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

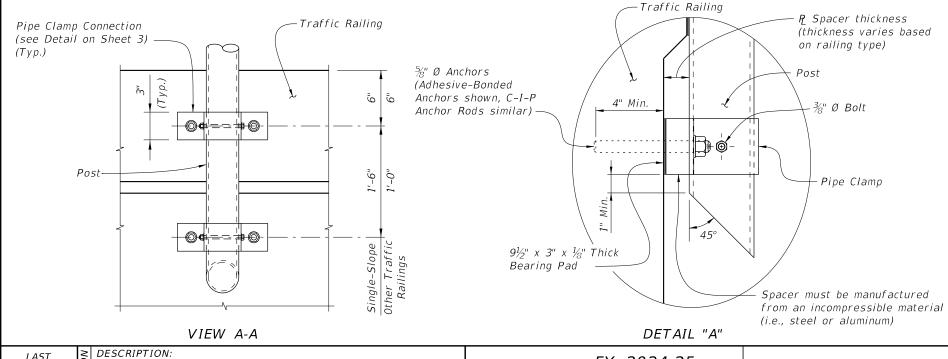
For Pull Post Assembly Detail for Traffic Railing see Sheet 3.

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TABLE OF CHAIN LINK FENCE COMPONENTS				
COMPONENT ASTM DESIGNATION		COMPONENT INFORMATION		
Posts	F1083	Galvanized Steel Pipe - $3\frac{1}{2}$ " NPS, Schedule 40 Regular Grade		
Chain Link Fabric (2" mesh with twisted top and knuckled bottom selvage)	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating		
	A491	Aluminum Coated Steel - 9 gage (coated wire diameter)		
	F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b		
Tie Wires	F626	Zinc Coated Steel Wire - 9 gage		
Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)		
Tension Bars F626 $\frac{3}{16}$ " (Min. thickness) x $\frac{3}{4}$ " (Min. width) x 6'-10" (Min. height) Steel Bars		$^{3}\!$		
Tension Bands	ension Bands F626 14 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands			
		Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)		
Tension Wire	A824 & A817	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating		
		Type I (Aluminum Coated Steel Wire) - 7 gage		
Hog Rings	F626	Zinc Coated Steel Wire - 12 gage		
Brace Rails	F1083	Galvanized Steel Pipe – $1\frac{1}{4}$ " NPS, Schedule 40 Regular Grade		

TABLE OF POST ATTACHMENT COMPONENTS				
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe Clamps		A36 or A709 Grade 36	¼" Steel ዊ	
Base Plates		A36 or A709 Grade 36	¾" Steel P	
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $\frac{3}{4}$ " Ø	
Spacers		-	Plate thickness varies based on traffic railing type (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Pipe (Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp Connections	
Washers		F 436	Flat Washers for Pipe Clamp Connections	
Bearing Pads (Plain Neoprene)		-	In accordance with Specification Section 932 for Ancillary Structures	



POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

WELDING:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

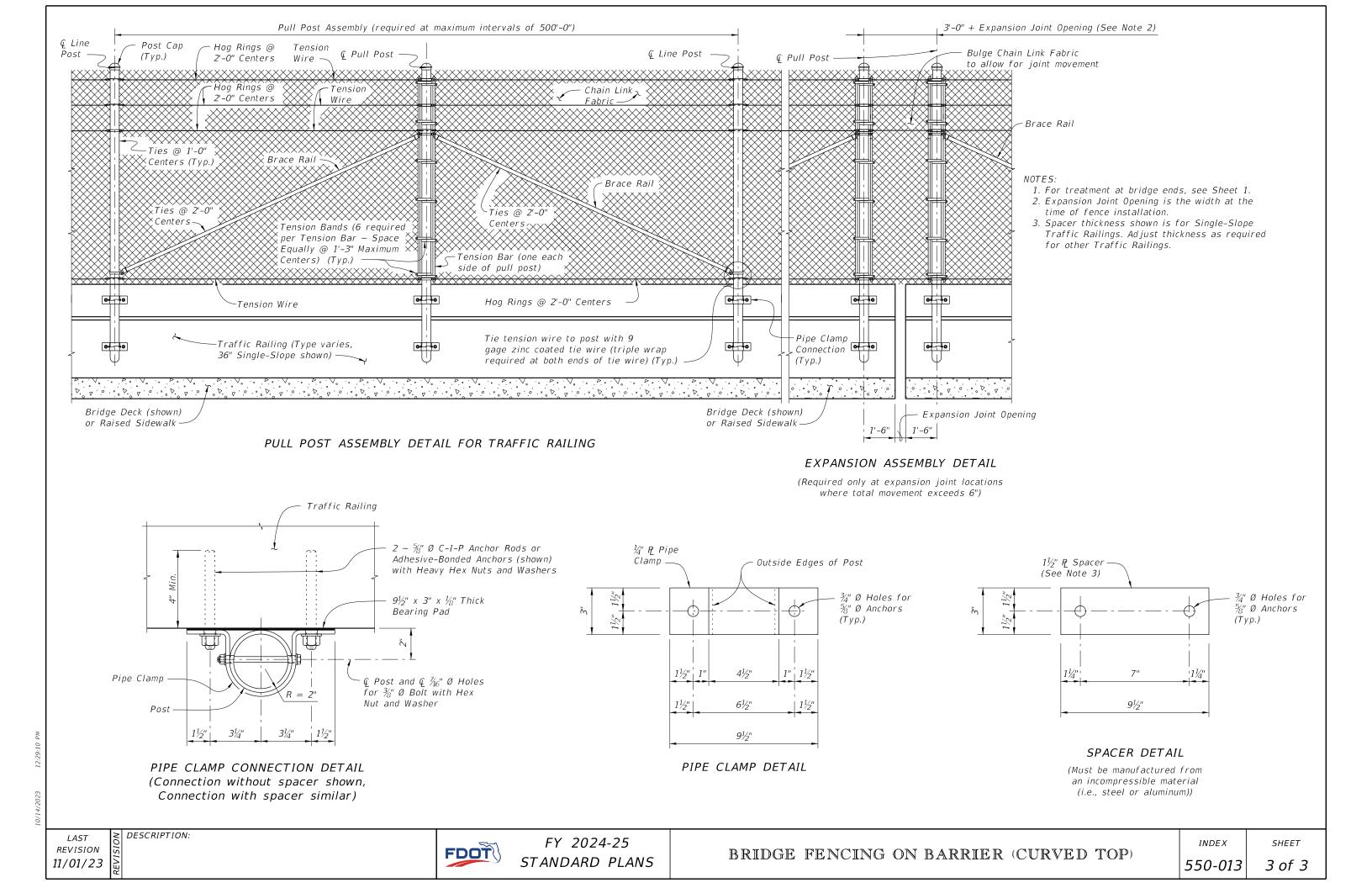
REVISION 11/01/23

FDOT

FY 2024-25 STANDARD PLANS

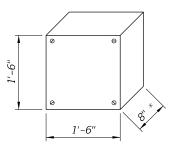
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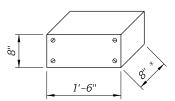


- 1. Furnish and install approved Conduits, Fittings and Embedded Junction Boxes (EBJ's) in accordance with Specification Sections 630 and 635, this Standard, the National Electric Code (NEC) and as directed by the Engineer.
- 2. Furnish and install Embedded Junction Boxes (EJB) with weatherproof covers sized in accordance with NEC requirements and the maximum size limits shown. Install EJB adjacent to the Begin and End of Bridges, Begin and End of Retaining Walls, (except omit EJB adjacent to the Bridge unless a precast Traffic Railing with junction slab is used), and at other locations as necessary to maintain 300 foot maximum spacing. See Plans for additional locations and details.
- 3. For Conduit not designated for future use, see Plans for details. For Conduit designated for future use, stub out and cap the Conduit. Drive a 3'-0"± long ¾" (min.) diameter Steel Pipe flush with the ground line adjacent to the end of the Conduit as shown on Sheets 2, 3 or 4. Provide the location of the stub out with Steel Pipe to the Engineer for inclusion on the As-Built Plans.
- 4. Shift vertical Railing reinforcement symmetrically to provide 2" clearance to EJB. Space shifted vertical reinforcement at minimum 3" centers. Cut horizontal Railing reinforcement to provide 2" clearance to EJB and provide supplemental reinforcement as shown. To facilitate placement of Conduit, Expansion Fittings, and Expansion/Deflection Fittings, shift reinforcing a maximum of 1" but do not cut railing reinforcing to facilitate Conduit or Fittings. Do not bundle Conduits, or Conduit and horizontal reinforcement.
- 5. Place conduits as indicated in this Standard unless Structures Plans indicate fewer.

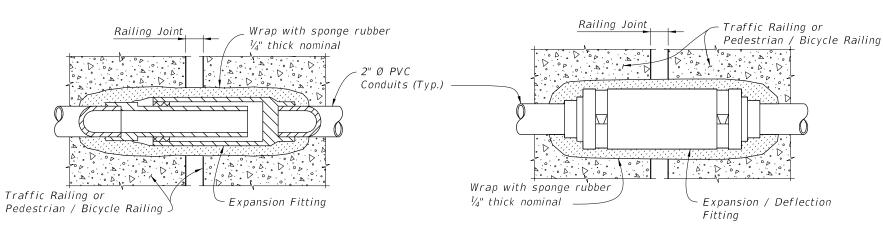
* Reduce to 6" maximum when installed in Pedestrian/ Bicycle Railings.



EJB "A" Double or Triple Conduit (Maximum Dimensions)



EJB "B" Single Conduit (Maximum Dimensions)



DETAIL "A" EXPANSION FITTING DETAIL DETAIL "B" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / CONCRETE) DETAIL "C" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / SOIL)

GENERAL

LAST REVISION 11/01/23

DESCRIPTION:

FDOT

FY 2024-25
STANDARD PLANS

CONDUIT DETAILS - EMBEDDED

Expansion / Deflection

3...

Fitting_

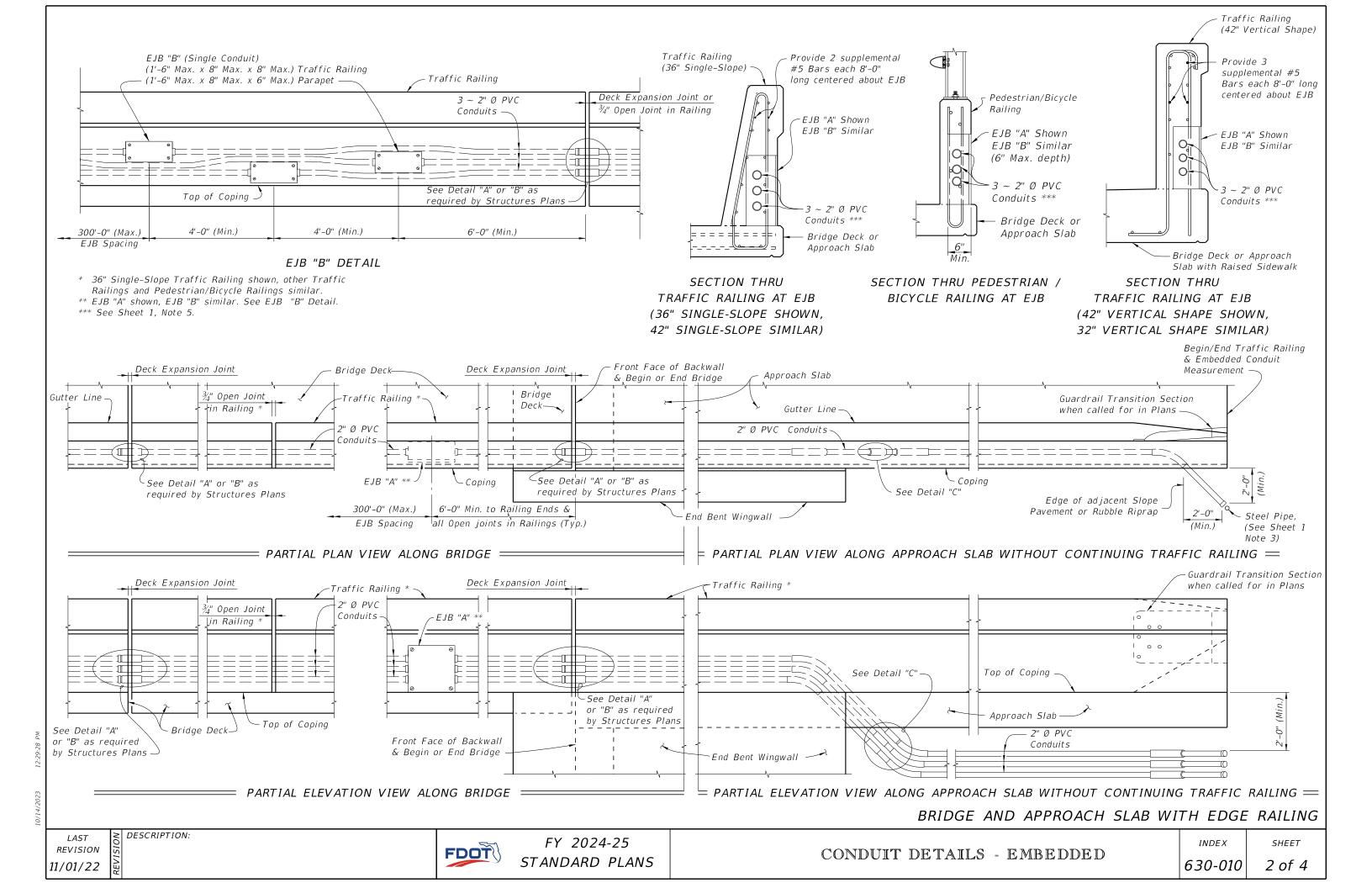
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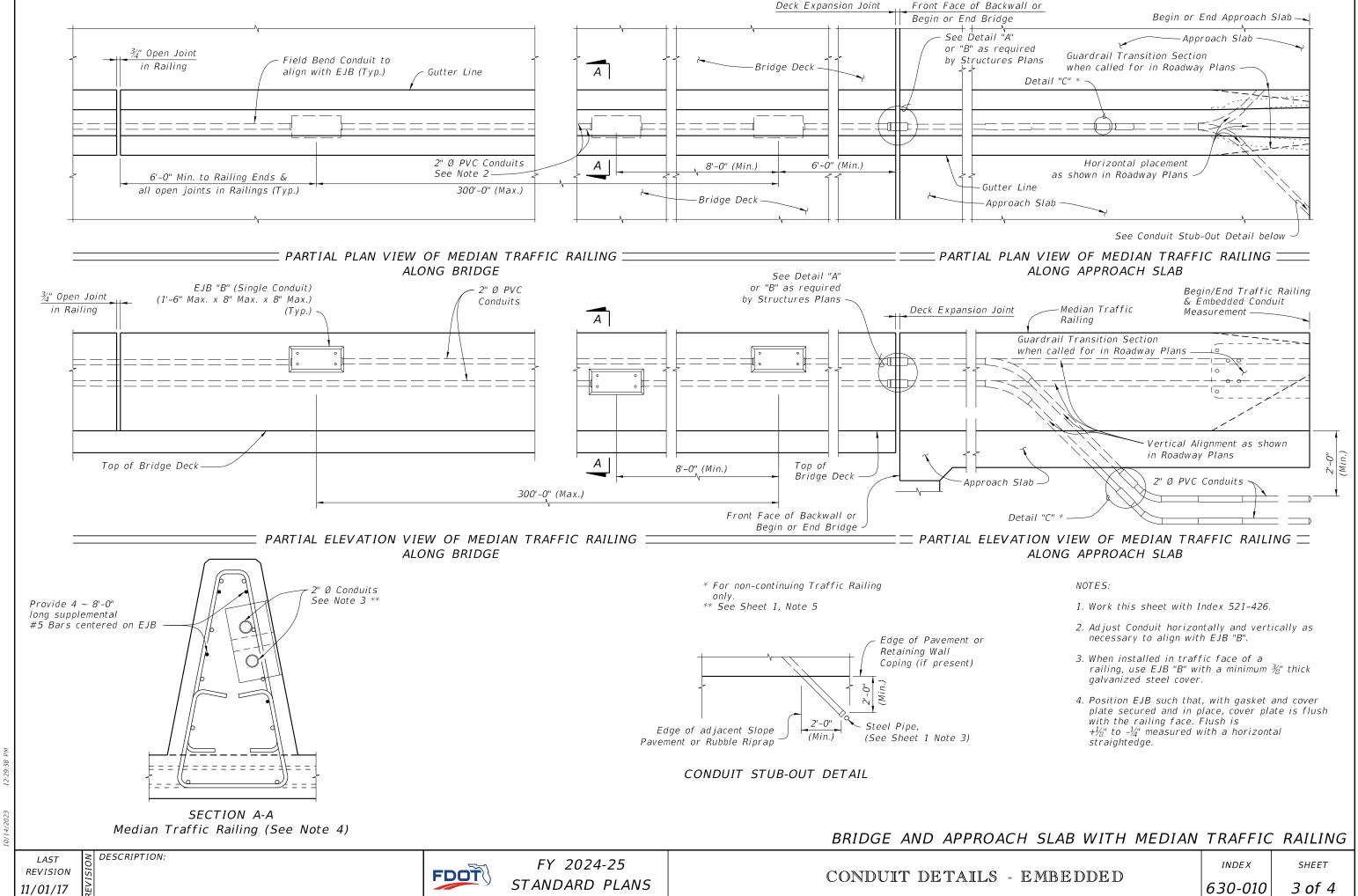
Wrap with sponge rubber

Ø PVC Conduits

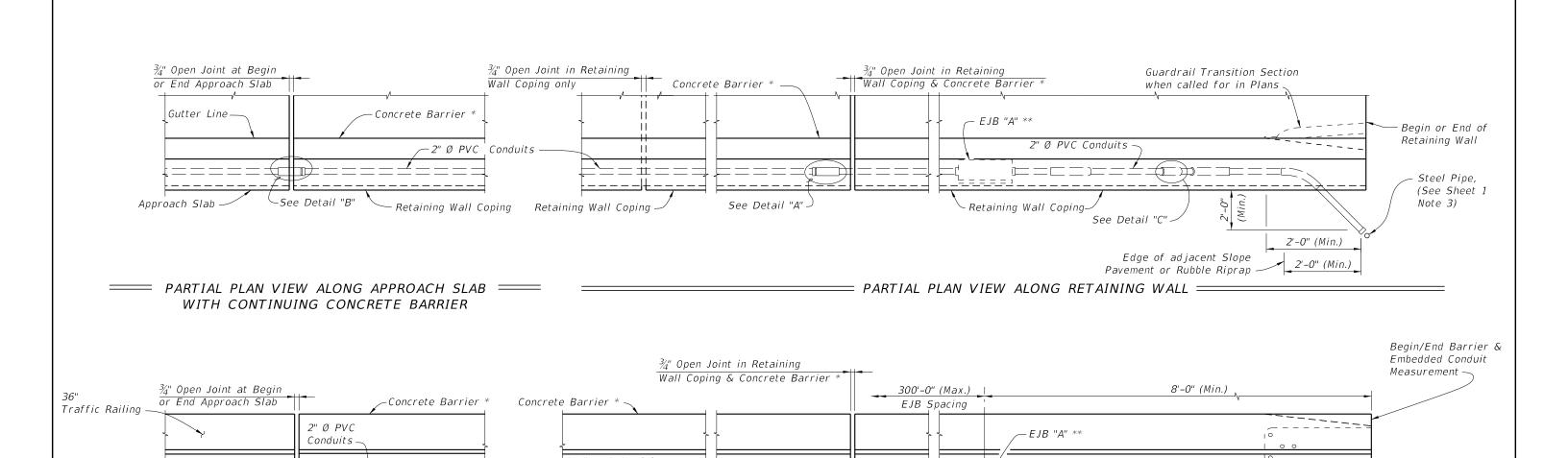
½" thick nominal

SHEET





3 of 4



See Detail "A"-

_ Retaining Wall Panels (Typ.) ノ

PARTIAL ELEVATION VIEW ALONG APPROACH = SLAB WITH CONTINUING Concrete Barrier (Retaining Wall Mounted Concrete Barrier shown, Traffic Railing similar)

-Top of Coping

= PARTIAL ELEVATION VIEW ALONG RETAINING WALL

Top of Coping

Retaining Wall Coping-

* Index 521-610 Concrete Barrier/Junction Slab shown, other railings and parapets similar.

** EJB "A" shown EJB "B" similar. See EJB "B" Detail on Sheet 2.

See Detail "B"

Approach Slab

APPROACH SLAB AND RETAINING WALL WITH CONCRETE BARRIER

REVISION 11/01/22

DESCRIPTION:

FDOT

3/4" Open Joint in Retaining

Wall Coping only

Retaining Wall Coping.

FY 2024-25 STANDARD PLANS

CONDUIT DETAILS - EMBEDDED

See Detail "C"

INDEX 630-010

SHEET

Shoulder Line

Guardrail Transition Section when called for in Plans

2" Ø PVC Conduits

- b. Conduit access through box girder end diaphragms with minimum 1" clearance in all directions.
- c. Conduit expansion fitting details.
- d. Fastener details for the interior electrical system.
- e. Single line diagram showing mini power centers, switches, contactors, timers, etc.
- f. Mini power center details including circuit breaker details.
- g. Mini power center mounting details if required.
- h. Feeder schedule.
- 2. Ensure installation meets all requirements of the latest edition of the National Electrical Code (NEC) and local ordinances. Install grounding in accordance with NEC Article 250. Maintain separation between 480V and 120V Conductors / Conduits
- 3. Furnish all labor, equipment, materials, and incidentals required for a complete and functional installation.
- 4. Use only new, unused and Underwriters Laboratories (UL) listed equipment and materials for outdoor use.
- 5. Furnish and install polyvinyl chloride (PVC) conduit in conformance with UL Section 651, NEC Section 347 and NEMA TC-2, UV-resistant and schedule 80. Bend conduits as necessary to connect to loads.
- 6. Provide PVC sleeve 2" larger in diameter than conduit to accommodate construction tolerance.
- 7. Install a UL labeled expansion fitting for specified PVC conduit at all structure expansion joints. Provide certification that the expansion fitting meets the following minimum requirements: Compatibility with the connected conduits, waterproof, UV protected and allows longitudinal movement equal to that of the Expansion Joint.
- 8. Use only Alloy 316 stainless steel supporting hardware. Provide minimum $\frac{3}{16}$ " Ø fasteners. For concrete or SIP form mounting, provide anchor bolts (expansion, drop-in or adhesive) suitable for dynamic loading (due to vibration caused by traffic). Install fasteners to avoid conflicts with reinforcing steel and PT ducts. For structural steel mounting, do not attach fasteners to main members, i.e. webs
- 9. Furnish power distribution at 480V AC, 1 phase, with step down transformers at regular intervals. Furnish 7.5 KVA mini power center with eight 20A breakers as the step down transformer, feeding a maximum of 20 lamps and 20 receptacles. Each mini power center will provide power to no more than 1000' of bridge, preferably 500' on each side of the mini power center, 480V top feed, 120V bottom feed to maintain separation.
- 10. Furnish and install lighting contactors to switch the 480V AC feeding the mini power centers.
- 11. Furnish and install copper conductors, Type XHHW. Do not use any conductor larger than #4 AWG.
- 12. Provide enough slack in all interior cable terminations to allow for minor shifting of the structure.
- 13. Furnish and install National Electric Manufacturers Association (NEMA) Type 4X (non-metallic) surface mounted boxes sized in conformance with the NEC.
- 14. Furnish and install 120V duplex receptacles (GFI, NEMA Type 5-20R), in non-metallic outlet boxes at 50' maximum on centers. Provide each receptacle with a gasketed weather-protective outdoor plate. Maximum wire size to connect to
- 15. Furnish and install surface mounted, fully enclosed, incandescent light fixtures with gasketed clear globes and wire guards at 50' maximum on centers. Provide 100 watt, 130 volt, vibration resistant and brass base incandescent lamps.
- 16. Provide six hour reset timers for each circuit to turn off the lighting system automatically.

CROSS REFERENCES:

- 1. For Maintenance Light Details, see Sheet 2.
- 2. For actual bridge section, see Structures Plans.

1 of 2

DESCRIPTION:

