

# FY 2022-23 STANDARD PLANS FOR BRIDGE CONSTRUCTION

Effective for Projects with Lettings in the Fiscal Year (FY) from

July 1, 2022 through June 30, 2023

FY 2022-23 Standard Plans for Road and Bridge Construction Topic No. 625-010-003 State of Florida Department of Transportation
Office of Design
Mail Station 32
605 Suwannee Street
Tallahassee, Florida 32399-0450

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20054	450-054	Florida-I 54 Beam - Standard Details	20660	455-060	60" Prestressed Concrete Cylinder Pile
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20072	450-072	Florida-I 72 Beam - Standard Details	20900	400-090	Approach Slabs (Flexible Pavement Approaches)
20078	450-078	Florida-I 78 Beam - Standard Details	20910	400-091	Approach Slabs (Rigid Pavement Approaches)
20084	450-084	Florida-I 84 Beam - Standard Details	Bridge Expai	nsion Joints	
20096	450-096	Florida-I 96 Beam - Standard Details	21100	458-100	Strip Seal Expansion Joint
20120	450-120	AASHTO Type II Beam	21110	458-110	Poured Joint With Backer Rod Expansion Joint System
20199	450-199	Build-Up & Deflection Data For Prestressed I-Beams	Structures A	access and Light	ting
20210	450-210	Typical Florida-U Beam Details and Notes	21200	521-660	Light Pole Pedestal
20248	450-248	Florida-U 48 Beam - Standard Details	21210	630-010	Conduit Details
20254	450-254	Florida-U 54 Beam - Standard Details	21220	510-001	Navigation Light System Details (Fixed Bridges)
20263	450-263	Florida-U 63 Beam - Standard Details	21240	715-240	Maintenance Lighting For Box Girders
20272	450-272	Florida-U 72 Beam - Standard Details	21250	460-250	Access Hatch Assembly For Steel Box Sections
20299	450-299	Build-Up and Deflection Data For Florida-U Beams	21251	460-251	Access Hatch Assembly For Concrete Box Sections
N/A	450-450	Florida Slab Beam Typical Details and Notes (New Index)	21252	460-252	Access Door Assembly For Concrete Box Sections
N/A	450-451	12" Florida Slab Beam (New Index)	Standard Bai	r Bending Detai	ils
N/A	450-452	15" Florida Slab Beam <b>(New Index)</b>	21300	415-001	— Standard Bar Bending Details <b>Renamed:</b> Bar Bending Details (Steel)
N/A	450-453	18" Florida Slab Beam (New Index)	N/A	415-010	Bar Bending Details (FRP) (New Index)

Design Standards Index	Standard Plans Index	Index Title			
Temporary D	etour Bridges				
21600	102-200	Temporary Detour Bridge General Notes and Details <b>Renamed:</b> Temporary Acrow 300 Series Detour Bridge General Notes and Details			
N/A	102-201	Temporary Acrow 700XS Series Detour Bridge General Notes and Details (New Index)			
21610	102-210	Temporary Detour Bridge Details - Timber Pile Foundations			
21620	102-220	Temporary Detour Bridge Details - Steel H Pile Foundations			
21630	102-230	Temporary Detour Bridge Details - Steel Pipe Pile Foundations			
21640	102-240	Temporary Detour Bridge Thrie-Beam Guardrail			
Post-Tension	ing				
21801	462-001	Post-Tensioning Vertical Profile			
21802	462-002	Post-Tensioning Anchorage Protection			
21803	462-003	Post-Tensioning Anchorage and Grouting Details			
Fender Syste	em Details				
21930	471-030	Fender System - Prestressed Concrete Piles			
22440	455-440	Precast Concrete CFRP/GFRP & HSSS/GFRP Sheet Pile Wall			
Square and I	Round Concrete	Piles (Corrosion Resistant)			
22600	455-101	Notes and Details for Square CFRP & SS Prestressed Concrete Piles			
22601	455-102	Square CFRP and SS Prestressed Concrete Pile Splices			
22612	455-112	12" Square CFRP and SS Prestressed Concrete Pile			
22614	455-114	14" Square CFRP and SS Prestressed Concrete Pile			
22618	455-118	18" Square CFRP and SS Prestressed Concrete Pile			
22624	455-124	24" Square CFRP and SS Prestressed Concrete Pile			
22630	455-130	30" Square CFRP and SS Prestressed Concrete Pile			
22654	455-154	54" Square CFRP and SS Prestressed Concrete Pile			
22660	455-160	60" Square CFRP and SS Prestressed Concrete Pile			

Standard Plans Index	Description
000-510	Sheet 1: Added directional arrows to SECTION AA; Updated the Table to match FDM; Added 2-Lane option to pavement with median to be consistent with FDM.
000-511	Sheet 1: Updated Note 4 to match values in FDM; Added ONE Lane option to the Facilities to be consistent with FDM.  Sheet 2: Update table to match FDM; Changed ratio in the PROFILE views for clarity.
102-600	Sheet 5: Changed the Height reference Note and deleted the 60" Width in the 3 POST SUPPORT MOUNTING DETAILS; Moved TEMPORARY SIGN SUPPORT NOTE 9 to TABLE 7, as Note 5; Deleted the 60x64 and the 120x60*Rectangle sign sizes and associated note from TABLE 7; Deleted "(See Note 7)" reference from the Diamond sign in TABLE 7.  Sheet 6: Rearranged all signs; Added signs: R4-11, W11-1, and W16-1p; Deleted all the "MOT" signs.  Sheet 7: Added the SIDE ROAD INTERSECTING THE WORK ZONE details from Index 102-606.
102-602	Sheet 1: Updated Note 3 to "Only the Road Work Ahead sign is required when the work operation is in place 60 minutes or less"; Updated Note 6 to read as work operation instead of temporary condition.; Deleted Note 8 and added new Note "When there is no paved shoulder, the "Worker" sign (W21-1) may be used instead of the "Shoulder Closed" sign (W21-5a)."  Sheet 2: Updated Note 9 references to Note 8.
102-603	Sheet 1: Changed Note 6 - "work zone" to "work operations"; Deleted Note 8B, and re-organized 8A; Deleted Note 9.
102-604	Sheet 1: Deleted Note 9 and added new Note "As an option to the "STOP" sign (R1-1) and Restricted Left/Right Turning Movement sign (R3-1 or R3-2), the "SIDE ROAD INTERSECTING THE WORK ZONE" flagging operation from Index 102-600 may be used."; Moved Flagger from the center of the westbound lane to the shoulder.  Sheet 2: Moved the Flagger and associated dimension on the Southern lanes of the detail, from the southbound lane to the northbound lane.
102-606	Sheet 1: Changed Note 5 - "work zone" to "work operation"; Deleted Note 8; Moved Side Street detail to 102-600.
102-607	Sheet 1: Note 4: Deleted "provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating" and changed "delete" to "omit".
102-613	<pre>Sheet 1: Changed Note 5 and 7 - "work zone" to "work operation";</pre>

Standard Plans Index	Description
102-615	Sheet 1: Changed Note 5 - "work zone" to "work operation"; Deleted Note 8. Sheet 4: Moved buffer space to not include TMA and added arrow board callout to TMA.
102-620	Sheet 1: Deleted Note 5. Sheet 2: Changed Note - "work zone" to "work operation".
102-628	Sheet 1: Changed Note 3 - "work zone" to "work operation"; Moved buffer zone and the associated dimension "B" before the Truck Mounted Attenuator in the work zone.  Sheet 3: Changed the Arrow Board Mode to "CAUTION" in the approach traffic lanes.; Changed WZ sign 20-5aR to W1-4L.
102-660	Sheet 2: Changed Note 4 - "work zone" to "work operations"; Added Note 5 - "Pedestrian Diversion Option 2 may only be used for work zone speeds of less than or equal to 35 mph and when called for in the Plans or as approved by an Engineer"; Changed title "TEMPORARY PEDESTRIAN WAY" to "PEDESTRIAN SPECIAL DETOUR"; Changed title "TEMPORARY PEDESTRIAN WAY DIVERTING TRAFFIC INTO THE TRAVELED WAY" to "PEDESTRIAN DIVERSION - OPTION 1"; Added new detail "PEDESTRIAN DIVERSION - OPTION 2".
102-661	All Sheets: Title Change to "BICYCLE FACILITY CLOSURES".  Sheet 1: Changed Note 4 - "work zone" to "work operation".  Sheet 2: Added Notes; Updated Symbols; Added Typical PCMS Display; Changed TEMPORARY BICYCLE DIVERSION to "BICYCLE SPECIAL DETOUR" and TEMPORARY BICYCLE WAY DIVERTING TRAFFIC INTO THE TRAVELED WAY to "BICYCLE FACILITY SHIFT (With Lane Closure)"; Added "BICYCLE FACILITY SHIFT (Work Zone Speed of 35 mph or Less)".
102-680	Sheet 1: Changed Note 6 - "work zone" to "work operation".
350-001	Sheet 1: Updated Note 3.B (Deleted "standard load transfer" and changed Spacing of #5 bars from 38"); Updated Note 7 to revise reference to Specification 350 and Specification 931; Correct line work to extend to the bottom of the pavement in the BUTT CONSTRUCTION JOINT details; Added "Relation of Dowels to Tie Bars" detail.
370-001	Sheet 1: Changed "Class I" to "Class II" in SECTION A-A; Revised Note 1 to read " For asphalt base, use four expansion joints, spaced at 15-ft, per Index 350-001."
425-060	Sheet 3: Changed "Class I" to "Class II" in Table 1. Sheet 4: Changed "Class I" to "Class II" in the SHALLOW DITCHES - INLET VIEW detail.

Standard Plans Index	Description		
425-061	Sheet 2: Remove reference to Dimension "D" in SPI and change text in SPI and Index to "Flume Length" to allow for this to be called for in the Plans. The "varies" callouts were deleted from the section references and note because the sections as shown are not variable.		
430-001	Sheet 4: Moved "Joining Mainline Pipe to Stub Pipe Details" and Notes 2 and 3 to the Standard Plans Instructions: Deleted Notes 5-7.		
430-010	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II". Sheet 2: Changed "Class I" to "Class II" in TABLE 2.		
430-011	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II".  Sheet 2: Changed "Class I" to "Class II" in the DIMENSION AND QUANTITIES FOR ONE U-ENDWALL TABLE and added TABLE 1 to the title.  Sheet 3: Changed "Class I" to "Class II" in the DIMENSION AND QUANTITIES FOR ONE U-ENDWALL TABLE and added TABLE 2 to the title.  Sheet 4: Changed "Class I" to "Class II" in the DIMENSION AND QUANTITIES FOR BAFFLES TABLE and added TABLE 3 to the title; Changed "Class I" to "Class II" in the DIMENSION AND QUANTITIES FOR ONE U-ENDWALL TABLE and added TABLE 4 to the title.  Sheet 5: Added TABLE 5 to the title of the TABLE OF DIMENSION AND QUANTITIES FOR ONE GRATE.		
430-012	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II".		
430-020	Sheet 1: Changed GENERAL NOTE 3 - Cast Toe Walls in place using Class II Concrete.  Sheet 2: Changed "Class I" to "Class II" in the TABLE; Corrected the Dimension "B" in the Plan View of the Straight Flare Detail to "P".		
430-030	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II". Sheet 2: Changed "Class I" to "Class II" in both Tables.		
430-040	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II". Sheet 2: Changed "Class I" to "Class II" in the DIMENSION AND ESTIMATED QUANTITIES TABLES.		
430-090	Sheet 1: Changed GENERAL NOTE 1 from "Class I" to "Class II".  Sheet 2: Changed "Class I" to "Class II" TABLE 1 - U-ENDWALL  DIMENSIONS AND QUANTITIES.		
440-002	Sheet 1: Changed GENERAL NOTE 2 from "Class I" to "Class II".		
450-010	<b>Sheet 1:</b> Updated language concerning N type strands in the notes for clarity.		

Standard Plans Index	Description		
450-199	Sheet 1: Changed the camber tolerance 1" from 1/2".		
509-070	Sheet 1: Renumbered Sheet 2: Renumbered Sheet 3: Moved the RAILROAD CROSSING AT TWO-LANE ROADWAY and the RAILROAD CROSSING AT MULTILANE ROADWAY details to the FDOT Design Manual (FDM); Moved the RELATIVE LOCATION OF CROSSING TRAFFIC CONTROL DEVICES detail to New Sheet 3; Moved the RAILROAD DYNAMIC ENVELOPE (RDE) PAVEMENT MARKING DETAIL and the RAILROAD CROSSING PAVEMENT MESSAGE detail to Index 711-001. Sheet 4: Renumbered; Added the RELATIVE LOCATION OF CROSSING TRAFFIC CONTROL DEVICES detail.		
509-100	Sheet 1: Changed "Class I" to "Class II" in the Front View of the ACTIVE STATE Detail.		
515-052	Sheet 5: Added knuckled selvage as an option to the chain-link railing option.		
515-062	Sheet 6: Added knuckled selvage as an option to the chain-link railing option.		
520-001	Sheet 1: (NEW SHEET) General Note and Overview.  Sheet 2: (Old Sheet 1) Renumbered, rearranged, and updated notes.  Sheet 3: (Old Sheet 2) Renumbered, rearranged and moved General Notes to new Sheet 1.		
520-005	Deleted notes; redrew details.		
520-010	Redeveloped Index; Redrew details; Rewrote Notes		
521-610	Sheet 1: Note 15: Added reference to Index 630-001 (Conduit Details - Embedded) for information on embedded junction boxes and conduits.		
521-650	Sheet 1: Note 5: Added reference to Index 630-010 (Conduit Details- Embedded) for information on embedded junction boxes and conduits.		
521-660	Sheet 1: Added a pedestal design to allow slip forming of the traffic railing. Reconfigured sheet layout.		
522-002	Sheet 1: Added Note 2.C. Sheet 4: Changed back of sidewalk in CR-D detail to accommodate 4'-0" minimum dimension.		
548-020	Sheet 1: Deleted alternative allowable wall type 2E from 2D.		
550-001	Sheet 1: Changed GENERAL NOTE 8 from "Class I" to "Class II".		

Standard Plans Index	Description		
630-001	Sheet 1: Added fiber optic cable route marker label detail; Revised General Note 2 to read "When sidewalk is damaged by conduit installation, replace entire sidewalk slab".		
641-010	Redeveloped Index Sheet 1: (NEW SHEET) General Note, Table of Contents, and added Concrete Pole Assembly. Sheet 2: (NEW SHEET) Service Pole P-IIA. Sheet 3: (NEW SHEET) Service Pole P-IIB. Sheet 4: (NEW SHEET) Pedestal Pole P-IIC. Sheet 5: Previously Sheet 3 - Updated Notes and Legend. Sheet 6: Previously Sheet 4 - Updated Notes and Legend. Sheet 7: Previously Sheet 5 - Updated Notes and Legend. Sheet 8: Previously Sheet 6 - Updated Notes and Legend. Sheet 9: Previously Sheet 7 - Updated Notes and Legend. Sheet 10: Previously Sheet 8 - Updated Notes and Legend.		
641-020	Sheet 1: Moved Note 3 to new Note 7; Added Note 7; Renumbered General Notes; Dashed the Identification Tag and the handhole in the CCTV POLE ASSEMBLY detail.  Sheet 2: Updated the Assembly detail to match Sheet 1; Realigned the PLAN VIEW detail to match the Pole direction; Dashed the Identification Tag and the Handhole in the ELEVATION detail.  Sheet 3: Updated the Assembly detail to match Sheet 1; Moved the Spiral wire note in the SPIRAL REINFORCING ELEVATION detail to the Notes and added See Note 1 to the Spiral Reinforcing call out; Dashed the Conduit Entry Holes in the POLE ELEVATION detail to be consent with the Pole's view.  Sheet 4: Updated the Assembly Detail to match Sheet 1; Added Note 5.  Sheet 5: Added Handhole to the CONCRETE CCTV POLE GROUNDING detail and to DETAIL "D"-SIDE VIEW detail.		
646-001	Changed Note 4 to remove concrete type since specified in Spec 646; Added Note 5 "As an alternative to the direct buried "Post Mounted" Pedestrian Detector Assembly shown below, the post may be installed on a transformer base. Use a transformer base included on the APL approved as an alternative to a "Post Mounted" assembly."; Updated the PEDESTAL MOUNTED detail – to show in the ADJACENT TO SIDEWALK detail "Tie Bars" callout as a #4 Stirrups equally spaced, with a 12" maximum spacing; Changed all references "4" Nominal Aluminum Pole" to "Nominal 4" Aluminum Post (Sch.40) (4.5" OD).		
649-010	Sheet 3: Changed weld on Handhole.		

Standard Plans Index	Description			
649-020	Sheet 1: Added Note 3 to the GENERAL NOTES; Renumbered Notes; Deleted Note B and Added Note C and D to new Note 6; Added the Cabinet Adapter Bracket and dashed the Handhole in the STEEL CCTV POLE ASSEMBLY detail.  Sheet 2: Updated the Assembly Detail to match Sheet 1; Dashed the Handhole in the ELEVATION detail.  Sheet 3: Updated the Assembly detail to match Sheet 1.  Sheet 4: Updated the Assembly detail to match Sheet 1. Update callout to "See Note" on Handhole Detail.  Sheet 5: Updated the Assembly detail to match Sheet 1.  Sheet 6: Dashed the Handhole in the CONCRETE CCTV POLE GROUNDING detail and added the Handhole to DETAIL "E"-SIDE VIEW detail.			
649-031	Sheet 1: Note 4 added optional other materials. Sheet 2: Added note 4. Sheet 3 and 4: Added bolt as option for splice connection. Sheet 6: Clarified that domed shape top cap is permissible.			
654-001	Sheet 1: Reorganized to show the Beacon Assembly adjacent to the sidewalk; Update Note 1 to reference Index 700-120 for pull box, conduit, wiring and grounding installation requirements; Updated Note 7; Change all references "4" Nominal Aluminum Pole" to Nominal 4" Aluminum Pole (Sch. 40) (4.5" OD); Updated anchor bolts; Updated callouts.  Sheet 2: (NEW SHEET) showing the Beacon Assembly in the Sidewalk Curb.			
659-010	Changed Note 2.B.b. updated width to greater than or equal to 8'.			

Standard Plans Index	Description			
695-001	All Sheets: Renumbered Sheets 1 through 7: Due to introduction of two new sheets, updated the total sheet number from 7 to 9.  Sheets 1, 2, 3, 4: Updated the name of the office from "Transportation Statistics" to "Transportation Data and Analytics".  Sheet 1: Added 12 Port Patch Panel, Managed Field Ethernet Switch, and Note 6 for installation.  Sheet 2: Added 12 Port Patch Panel, Managed Field Ethernet Switch, and Note 6 for installation.  Sheet 4: Changed color scheme to vendor provided color scheme Sheet 6: (NEW SHEET) described the quartz piezoelectric weigh-in-motion installation for Type I Configuration.  Sheet 7: (NEW SHEET) described the quartz piezoelectric weigh-in-motion installation for two distinct Type III Configurations. Added a note to contact the Transportation Data and Analytics office for correct layout based on vehicle classification unit.  Sheet 6 (NEW SHEET 8): Updated Elevation View to match foundation details on Sheet 7 (NEW SHEET 9); Changed "4" Nominal Aluminum Pole" to ""Nominal 4" Aluminum Pole (Shc. 40) (4.5" OD)"; Added Solar Panel callout.  Sheet 7 (NEW SHEET 9): Updated Foundation Details to show bars and stirrups; Updated note 2 to reference Spec 646; Updated Note 6; Changed all references "4" Nominal Aluminum Pole" to "Nominal 4" Aluminum Pol			
700-010	Sheet 4: Changed NOTE 1 from "Class I" to "Class II".			
700-011	Sheet 2: Changed "Class I" to "Class II" in the BASE AND FOUNDATION DETAIL.			
700-020	Sheet 2: Changed "Class I" to "Class II" in the FOUNDATION Detail.			
700-040	Sheet 2: Changed the 2'-2" lap splice for the #5 tie bars on the spread footing plan view to 1'-8". Revised Note 5 to read "After galvanizing, provide magnetic particle testing on 100% of upright fillet welds."			
700-041	Updated Note 5 to read "After galvanizing, provide magnetic particle testing on 100% of upright fillet welds."			
700-091	Sheet 1: Updated GENERAL NOTE 6 with more specific language to include hot dip galvanized safety gate and to install per manufacturer's instructions.			
700-101	Added offset from median or island nose to Case VIII Detail.			

Standard Plans Index	Description		
700-102	Sheet 5: Updated FTP-38-06 fort, dimension, and numbering; Changed the fine amount on FTP-40-06.  Sheet 6: Changed the fine amount on FTP-41-06.  Sheet 10: Added FTP-90-22 and FTP-91-22; Updated MOT-1-06, MOT-4-06, MOT-5-06 MOT-6-06, MOT-7-06, MOT-8-06, and MOT-9-06 for size, dimension, and numbering; Shifted to accommodate new signs; Moved MOT-8-06 (MOT-8-22) and MOT-9-06 (MOT-9-22) to Sheet 11.  Sheet 11: Updated MOT-10-06 font size, dimension, and numbering; Shifted to accommodate new signs; Moved G20-1 and G20-2 to Sheet 12.  Sheet 12: Shifted signs: Added MOT-26A-22 and MOT-26B-22.		
700-109	Deleted Index.		
700-120	All Sheets: Renumbered Sheet 1: Updated General Note 4; Change all references "4" Nominal Aluminum Pole" to "Nominal 4" Aluminum Pole (Sch. 40) (4.5" OD)"; Updated anchor bolts. Deleted the ground wire from the FRONT VIEW in the POWER CONFIGURATION 'B' - WITHOUT AUXILIARY POLE.  Sheet 2: Added new detail to show foundation reinforcement; Change all references "4" Nominal Aluminum Pole" to "Nominal 4" Aluminum Pole (Sch. 40) (4.5" OD)"; Updated anchor bolts. Deleted the ground wire and strain relief fitting from DETAIL "B".  Sheets 3 through 11: Updated callouts to point to both details; Changed all references "4" Nominal Aluminum Pole to "Nominal 4" Aluminum Pole (Sch. 40) (4.5" OD)"; Updated anchor bolts; Added Note "Foundation reinforcement not shown".  Sheet 9: Changed "Point to Point Microwave" to "Wireless" from the FRONT VIEW and from Note 2; Removed "Microwave" from the FRONT VIEW and from Note 2.  New Sheet 10: Added Roadside Sign Assembly-8, Blank Out Signs. Sheet 10: Renumbered as Sheet 11.		
706-001	Sheet 3: Added - Note 3: "Use epoxy adhesive to install RPMs to concrete median nose curbs"; Added Note 4: "Install RPMs on clean, unpainted concrete surface. Do not paint curb surface where RPMs will be placed"; Changed all references to Yellow Reflective Durable Paint-Yellow.  Sheet 4: Revise Detail "G" to remove yellow paint from nose and call for white RPMs to be consistent with MUTCD.		

Standard Plans Index	Description		
711-001	All Sheets: Renumbered Sheet 1: Deleted Note 1 for Pavement Messages; Changed Note 5 (New Note 4) to "All pavement messages must be white except Route Shields and In Pavement Warning Markings"; Deleted Pavement Message Spacing Table; Deleted the General Notes; Added Pavement Warning Marking.  Sheet 2: Moved the MARKING FOR MERGE detail from Sheet 8.  Sheet 8: Deleted Right Turn Lane Drop and Island Details and DETAIL "C" from Index. Moved to the FDM, Combined with Signing Details shown in 230-5; Deleted the Traffic Channelization at Gore detail Note; Moved the Instructions associated with the TWO-WAY LEFT TURN LANE details to the SPI.  Sheet 9: Deleted Schemes for Transition details and DETAIL "D" from Index. Moved to FDM, Combined with Signing Details shown in Exhibit 230-05; Changed Detail "E" to " Markings for Merge" and Deleted the NOTE; Moved Details to Sheet 2; Moved the "Markings for Traffic Separation" to Sheet 8; Deleted Sheet 9.  Sheet 11: New Sheet 10: Revert to previous FY 2018-19 Index and delete redundant information shown in FDM Exhibit 212-1.  Sheet 12: (New Sheet 11) Deleted Note 4; Changed reference to "See Note 5" in the UNIVERSAL SYMBOL OF ACCESSIBILITY detail.  Sheet 14: (NEW SHEET) Added RAILROAD DYNAMIC ENVELOPE (RDE) PAVEMENT MARKING DETAILS and RAILROAD CROSSING PAVEMENT MESSAGE from Index 509-070 and RAILROAD CROSSING PAVEMENT MESSAGE from Index 509-070.		
711-003	Sheet 1: Added 18" white Chevrons to both entrance ramps. Dotted line for lane extension to extend to end of taper.  Sheet 2: Added 18" white Chevrons to both entrance ramps. Dotted line for lane extension to extend to end of taper.  Sheet 5: Added 18" white Chevrons to entrance ramp gore; Moved Detail "C" from sheet 5 to Sheet 3 of Index 706-001; Moved Note 1 to the Wrong-Way Arrow callout at the Gore; Deleted Note 2; Deleted the "See Note 3" reference from the Yellow Post Mounted Delineator callout.  Sheet 6: Moved Note 1 to the Wrong-Way Arrow callout at the Gore; Deleted the note reference and the Detail "C" reference for the Wrong-Way Arrow callout on the off ramp.		
715-001	Sheet 1: Updated fuse holder slug (blank) requirement from solid copper to manufacturer's suggested in three drawing locations.		

Standard Plans Index	Description
715-002	Sheets 1, 4, & 7: Changed general concrete requirement from Class I to Class II.  Sheet 2: Added clarification to callout. Wildlife-Sensitive lighting may also be used with standard mounting heights 30 feet through 50 feet (original intent).
715-003	NEW INDEX: "Utility Conflict Pole", light pole with a 15' rise X 16' length arm.

Work this Index with Index 102-210, 102-220, 102-230 and 102-240.

#### STRUCTURAL STEEL:

Steel Plates and Rolled Sections shall be ASTM A709 Grade 36. Pipe piles shall be ASTM A252 Grade 2, Fy = 35 ksi.

#### BOLTS, LAG SCREWS AND THREADED BOLT STOCK:

Furnish high strength bolts in accordance with ASTM F3125 Grade A325 Type 1. Furnish Threaded Stock in accordance with ASTM A36. Furnish Lag Screws in accordance with ASTM A307. Furnish steel washers and nuts compatible with Bolts, Threaded Stock and Lag Screws.

#### TIMBER AND LAGGING:

Timber and Lagging shall be No. 1 Southern Yellow Pine.

#### BACKWALL BENT PILES:

#### Timber Piles:

10' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6). Ultimate Capacity greater than 18 tons.

Splices are not allowed on any timber piles.

#### H-Piles:

12' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6).

Ultimate Capacity greater than 18 tons.

Shims admissible between backwall pile and cap. Test piles are not required for backwall piles.

#### **EXPANSION BEARINGS:**

Inspect the PTFE (Teflon) layer and stainless steel plate prior to installation. Do not use bearings that have a severely damaged or unbonded PTFE layer. Clean PTFE of all grit and grime prior to installation. Clean Stainless steel plate of all grit and grime prior to installation and finish to a smooth buffed surface.

#### DISTRIBUTING BEAMS:

Longitudinal stops restraining the distributing beams may be lengthened or shortened to center the distributing beam bearing on the cap beam.

The longitudinal stops are to bear on the distributing beam end frame.

#### **EXPANSION JOINT SETTINGS:**

Install the expansion joint considering the total continuous bridge length, location of fixed bearings and ambient temperature at the time of installation, assume a 1" expansion joint opening at 70 degrees F.

#### STORAGE FACILITY:

Contact
FDOT Statewide Aluminum Shop
2590 Camp Rd.
Oviedo, Fl.
407-278-2727

For shipping weights and dimensions of Temporary Bridge elements.

#### SHIPPING WEIGHTS AND DIMENSIONS:

#### Decking Sizes:

Type	Length	Width	Weight (lbs.)
Curb	5'	6'-9"	800
Curb	1 O'	6'-9"	1420
Curb	15'	6'-9"	2200
Curb	20'	6'-9"	2800
NonCurb	5'	5'-3''	650
NonCurb	10'	5'-3''	1000
NonCurb	15'	5'-3''	1600
NonCurb	20'	5'-3"	2100

Shipping weights and dimensions of other bridge components can be referenced in "Acrow Panel Bridging, Series 300, Technical Handbook".

#### TRAFFIC RAILING NOTES:

See Index 536-001 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements of AASHTO M180, Type II (Zinc coated).

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Do not drill Temporary Bridge components to attach Guardrail. Guardrail Bolts shall be placed between Truss members as shown in Index 102-240.

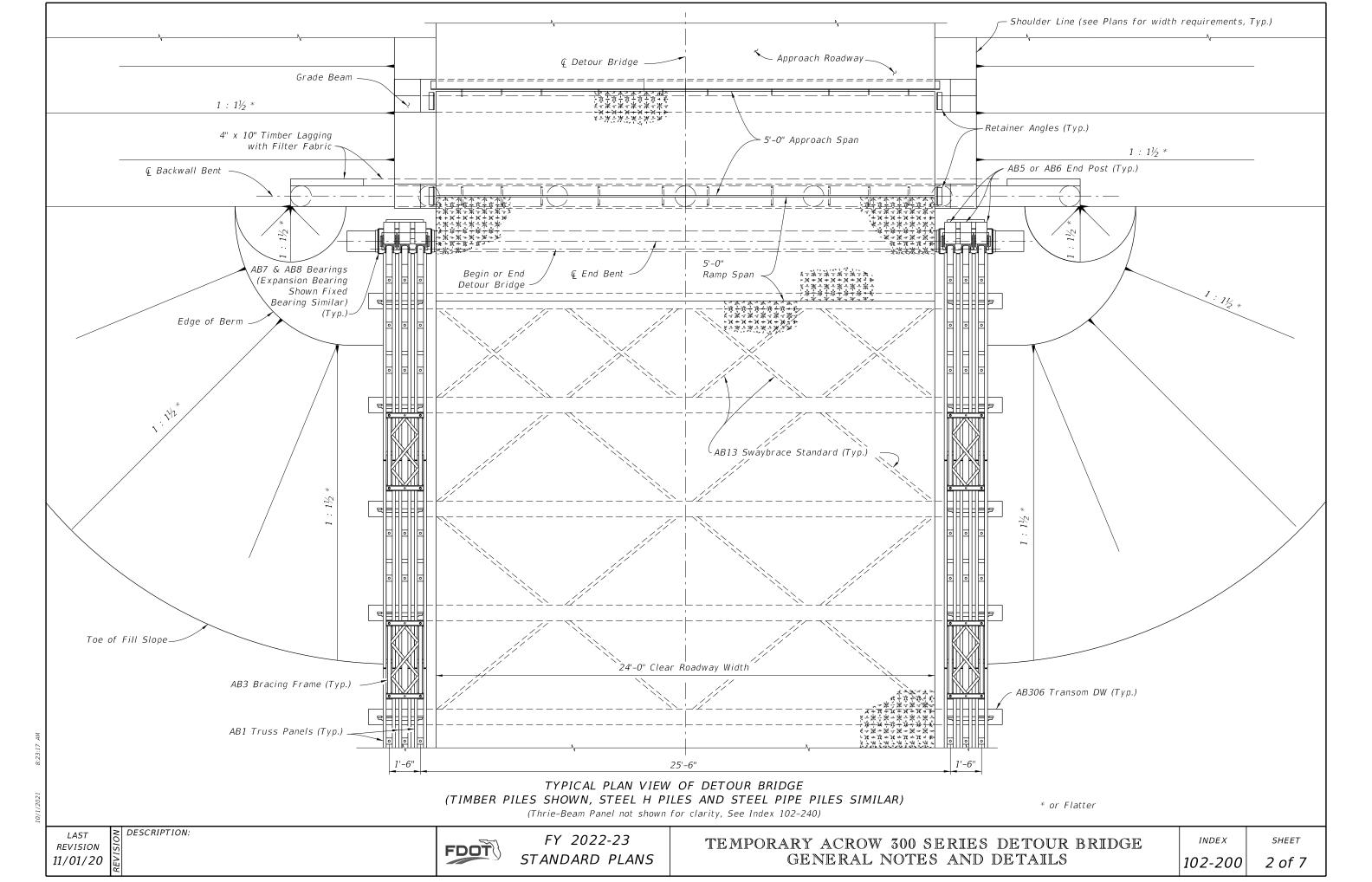
COATINGS: All Nuts, Bolts, Anchors, Washers and Backer Plates shall be hot-dip galvanized in accordance with the Specifications.

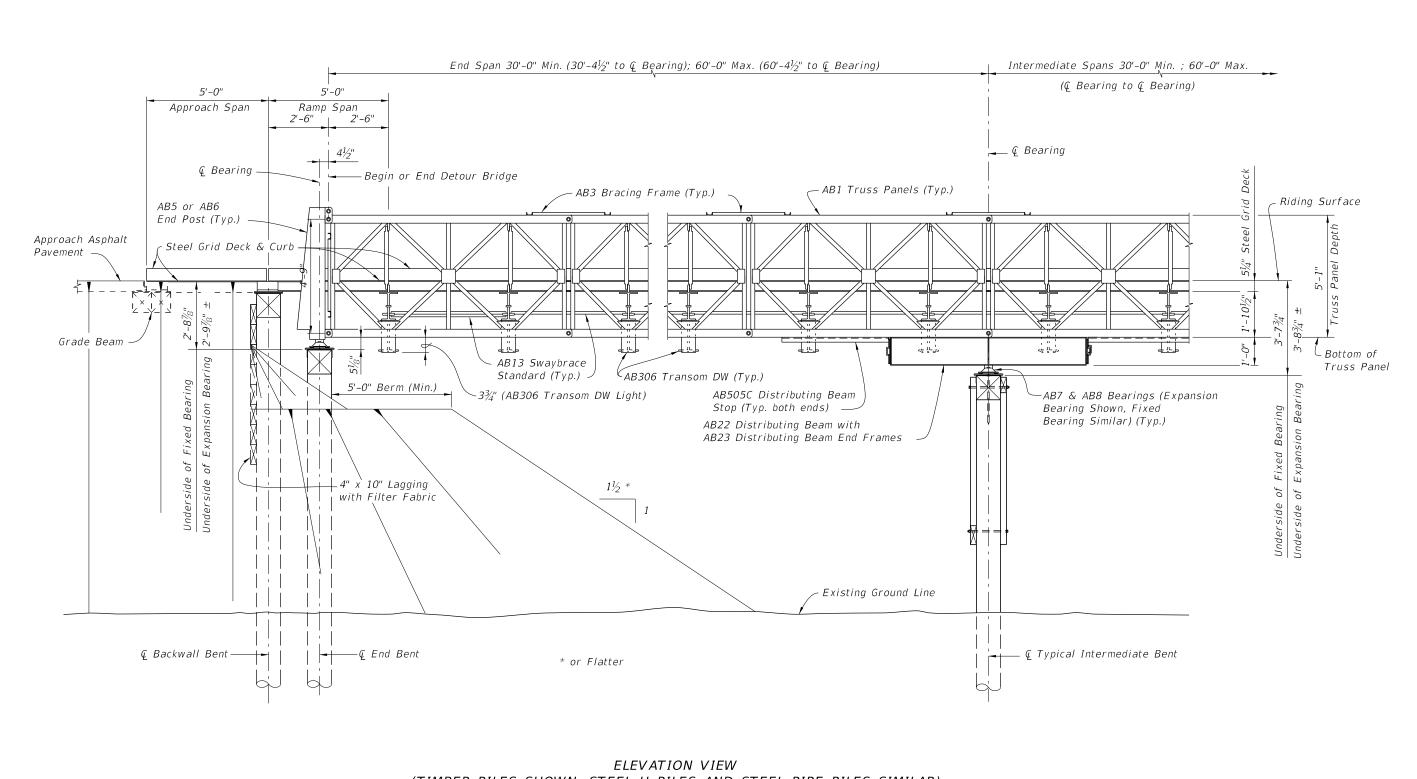
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered  $(\pm 1/4")$ .

#### PAYMENT:

Temporary Detour Bridge is to be paid for under Contract Unit Price for Special Detour. If a temporary bridge system other than that shown herein is used, the Contractor is responsible for renting or purchasing their own system. Payment for Temporary Guardrail work and Transition Block will be made under Pay Item Temporary Guardrail, LF.

Furnish and install Bridge Thrie-Beam Panels and all associated hardware as shown. Payment will be made with the Temporary Detour Bridge under the Pay Item Special Detour, LS. Turn over Bridge Thrie-Beam Panels and all associated hardware to the Department with the Detour Bridge components per Specifications Section 102-6.





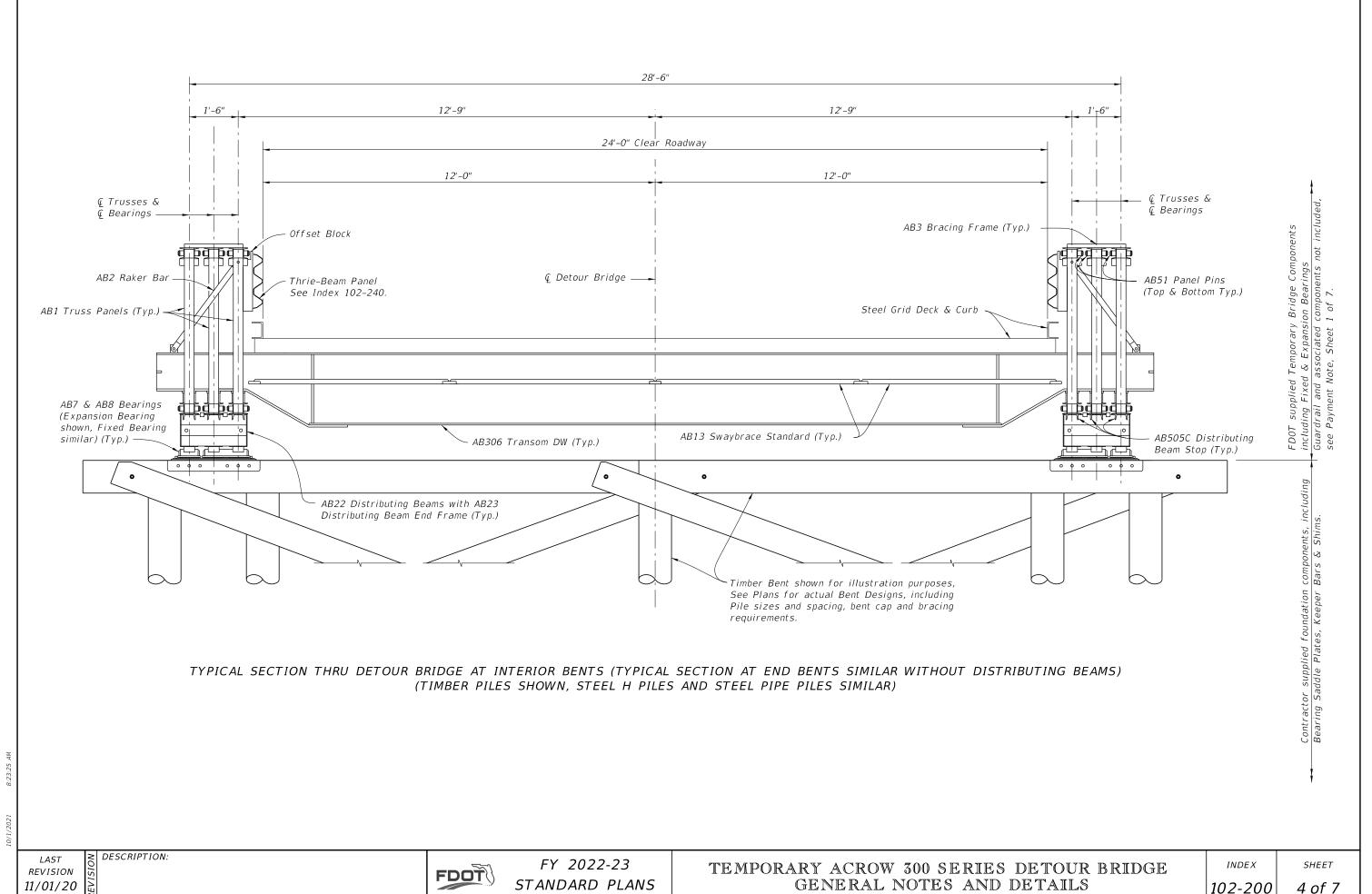
# (TIMBER PILES SHOWN, STEEL H PILES AND STEEL PIPE PILES SIMILAR)

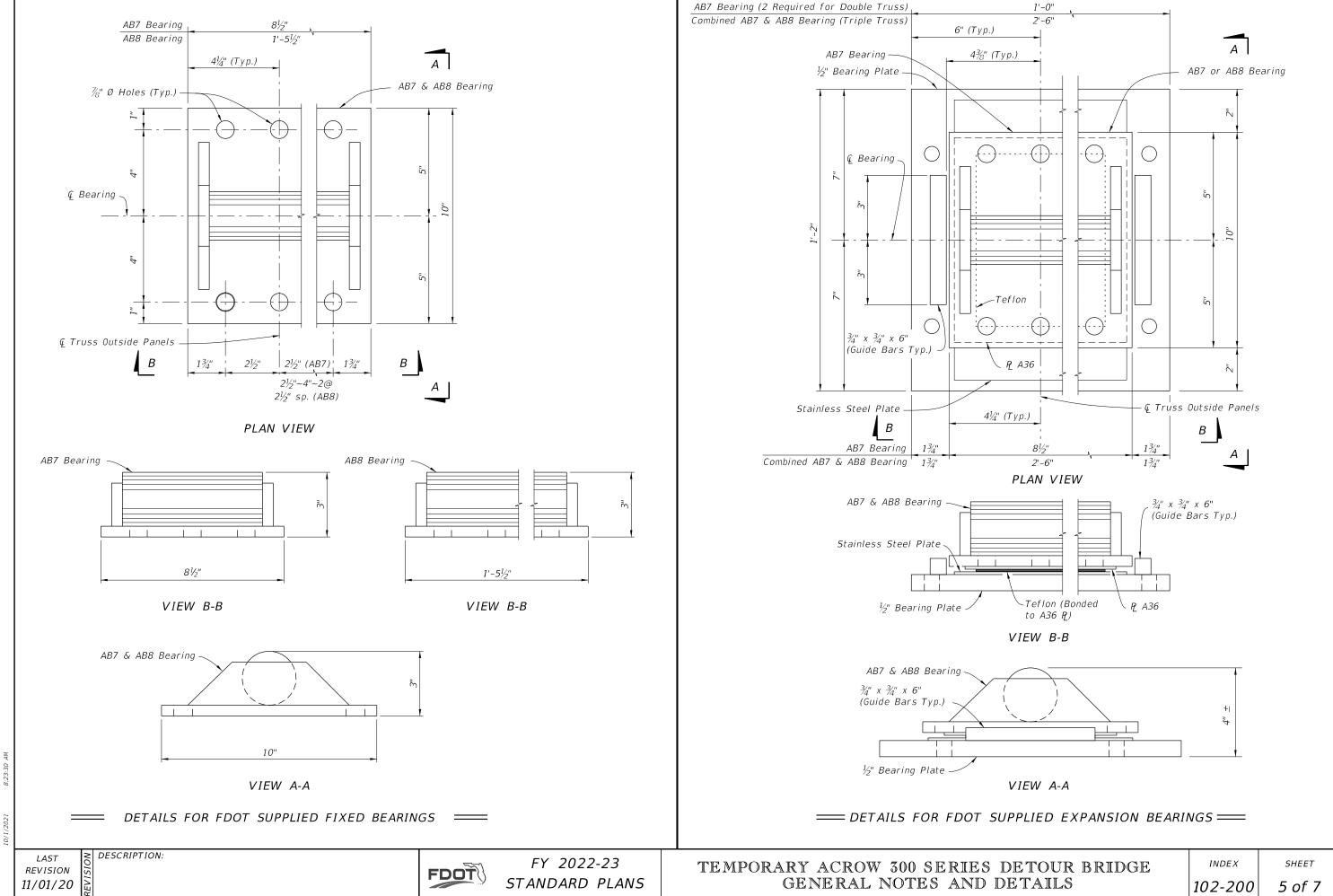
(Thrie-Beam Panel not shown for clarity, See Index 102-240)

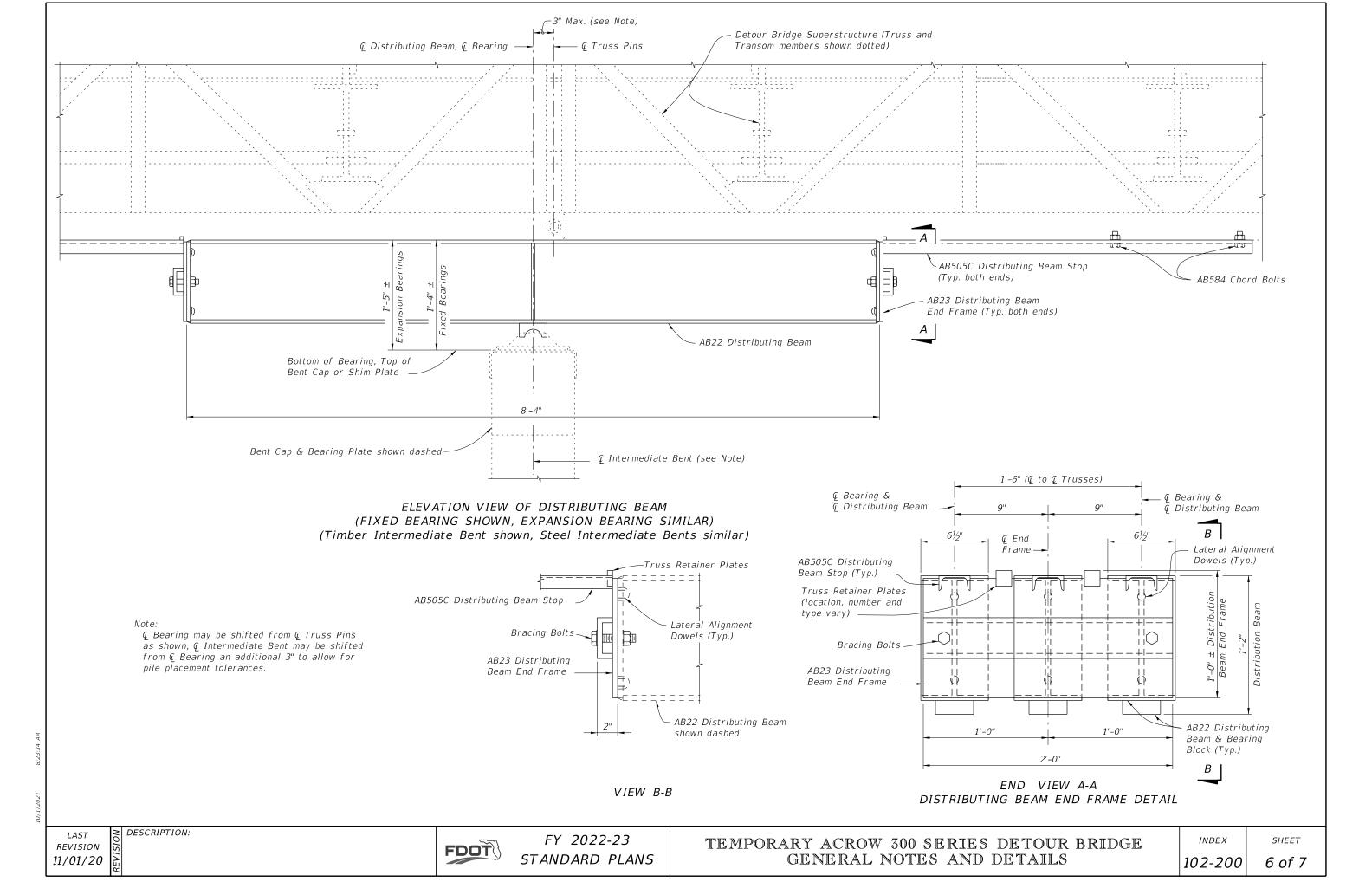
REVISION 11/01/20

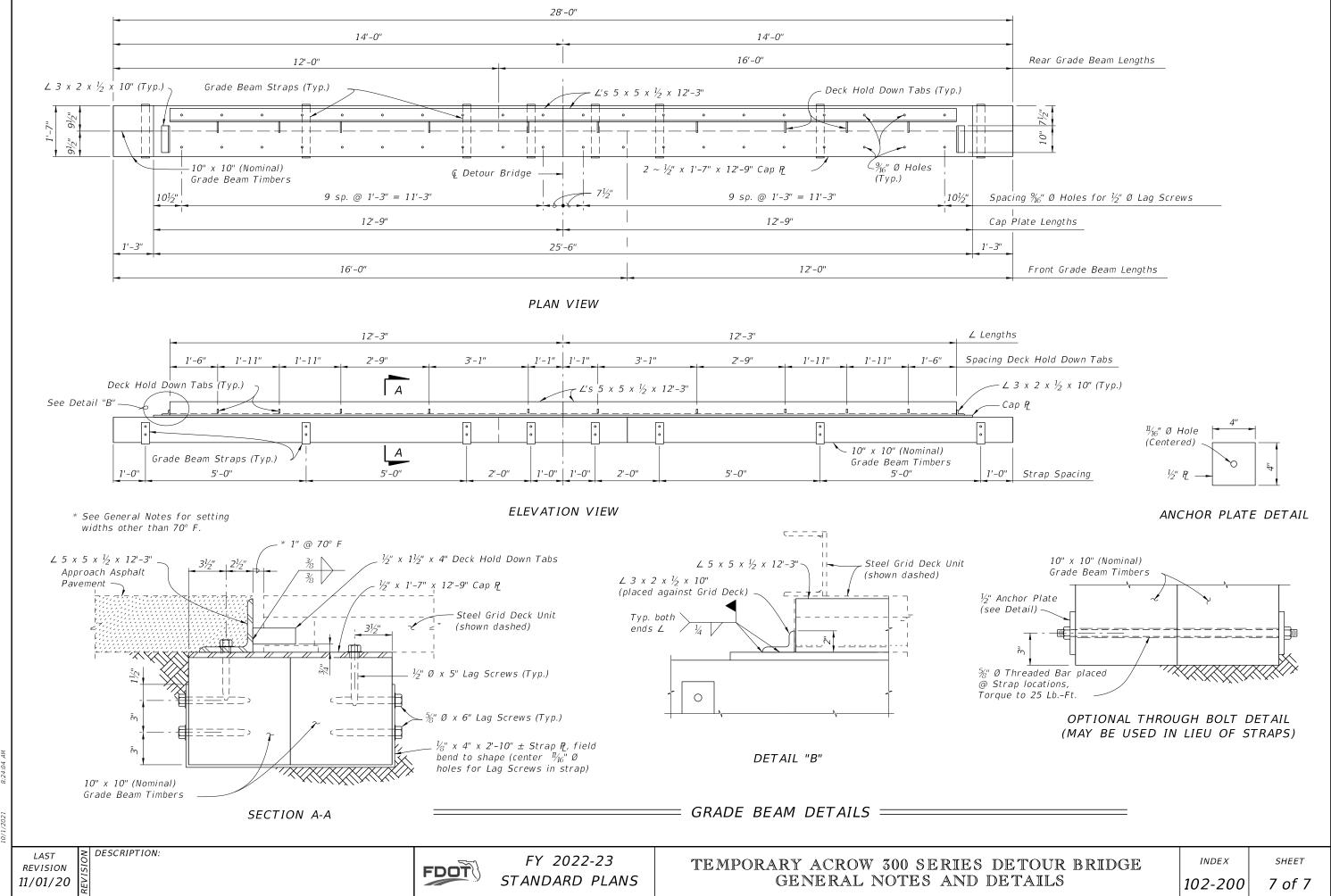
DESCRIPTION:

FDOT









Work this Index with Index 102-210, 102-220 and 102-230.

#### STRUCTURAL STEEL:

Steel Plates shall be ASTM A709 Grade 36.

#### **EXPANSION BEARINGS:**

Inspect the PTFE (Teflon) layer and stainless steel plate prior to installation. Do not use bearings that have a severely damaged or unbonded PTFE layer. Clean PTFE of all grit and grime prior to installation. Clean Stainless steel plate of all grit and grime prior to installation and finish to a smooth buffed surface.

#### DISTRIBUTION BEAMS:

Distribution beam stops restraining the distribution beams may be lengthened or shortened to center the distributing beam bearing on the cap beam.

The longitudinal stops are to bear on the distributing beam end frame.

#### **EXPANSION JOINT SETTINGS:**

Install the expansion joint considering the total continuous bridge length, location of fixed bearings and ambient temperature at the time of installation, assume a 2" expansion joint opening at 70 degrees F, (Expansion joint depends on span/bridge length and configuration).

#### STORAGE FACILITY:

Contact

FDOT Statewide Aluminum Shop 2590 Camp Rd. Oviedo, Fl. 407-278-2727

For shipping weights and dimensions of Temporary Bridge elements.

Contractor to coordinate with Storage Facility and Acrow to obtain required parts list.

Shipping weights and dimensions of other bridge components can be referenced in "Acrow Panel Bridging, Series 700XS, Technical Handbook".

#### APPROACH TRAFFIC RAILING NOTES:

See Index 536-001 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements of AASHTO M180, Type II (Zinc coated).

#### BOLTS, NUTS AND WASHERS:

Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Do not drill Temporary Bridge components to attach Guardrail. Guardrail Bolts shall be placed between Truss members as shown in Index 102-240.

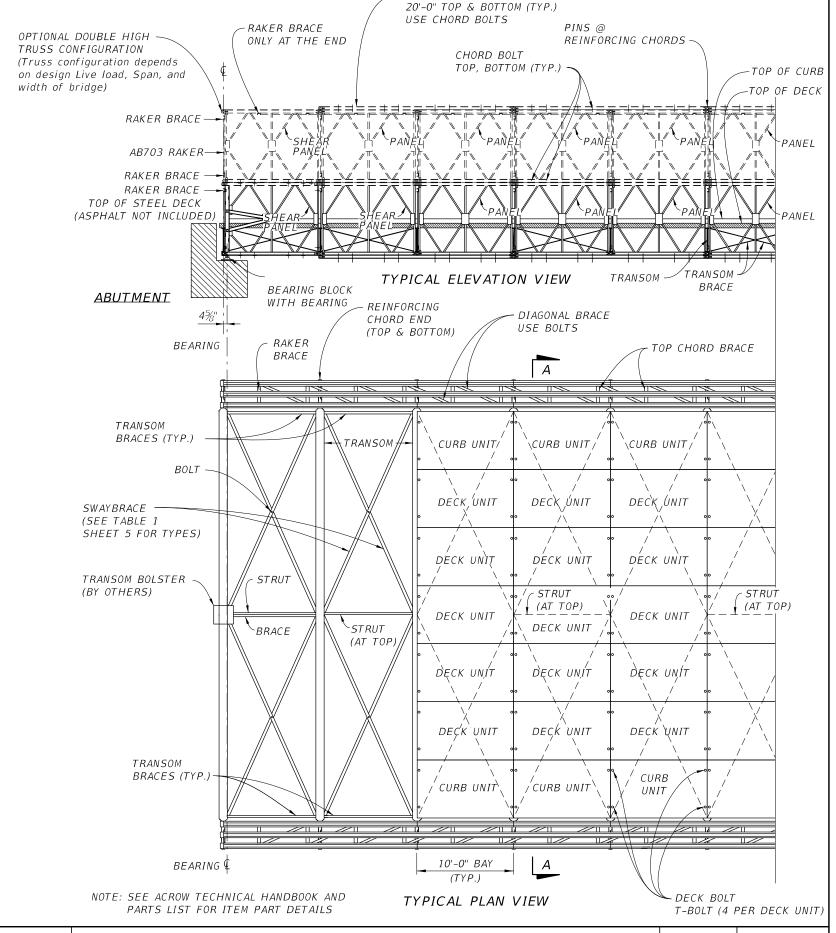
COATINGS: All Nuts, Bolts, Anchors, Washers and Backer Plates shall be hot-dip galvanized in accordance with the Specifications.

WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered  $(\pm 1/4")$ .

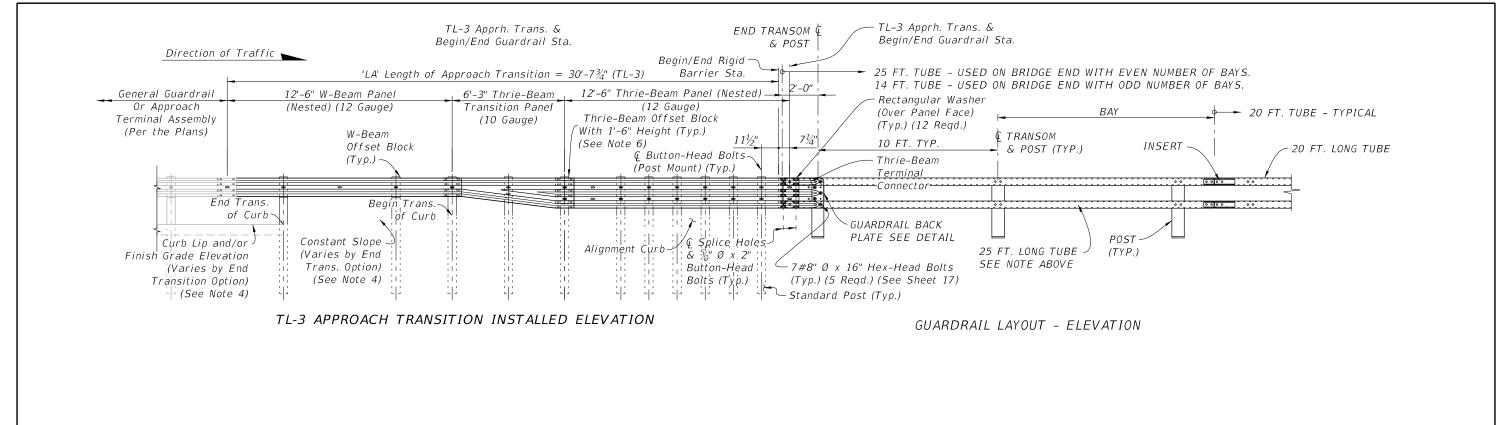
#### PAYMENT:

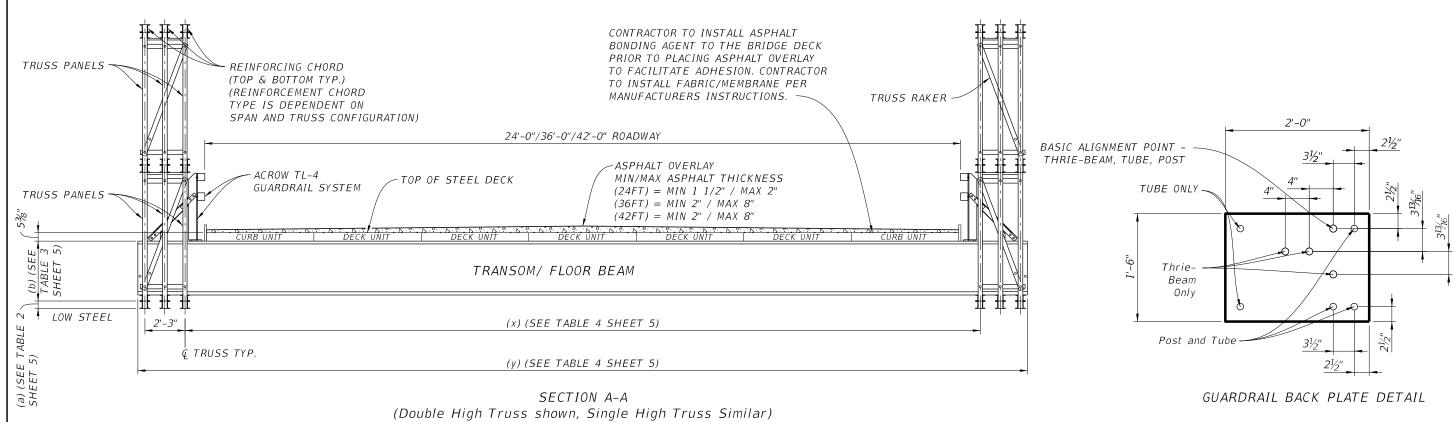
Temporary Detour Bridge is to be paid for under Contract Unit Price for Special Detour. If a temporary bridge system other than that shown herein is used, the Contractor is responsible for renting or purchasing their own system. Payment for Temporary Guardrail work and Transition Block will be made under Pay Item Temporary Guardrail, LF.

Furnish and install Bridge Thrie-Beam Panels and all associated hardware as shown. Payment will be made with the Temporary Detour Bridge under the Pay Item Special Detour, LS. Turn over Bridge Thrie-Beam Panels and all associated hardware to the Department with the Detour Bridge components per Specifications Section 102-6.



REINFORCING CHORD





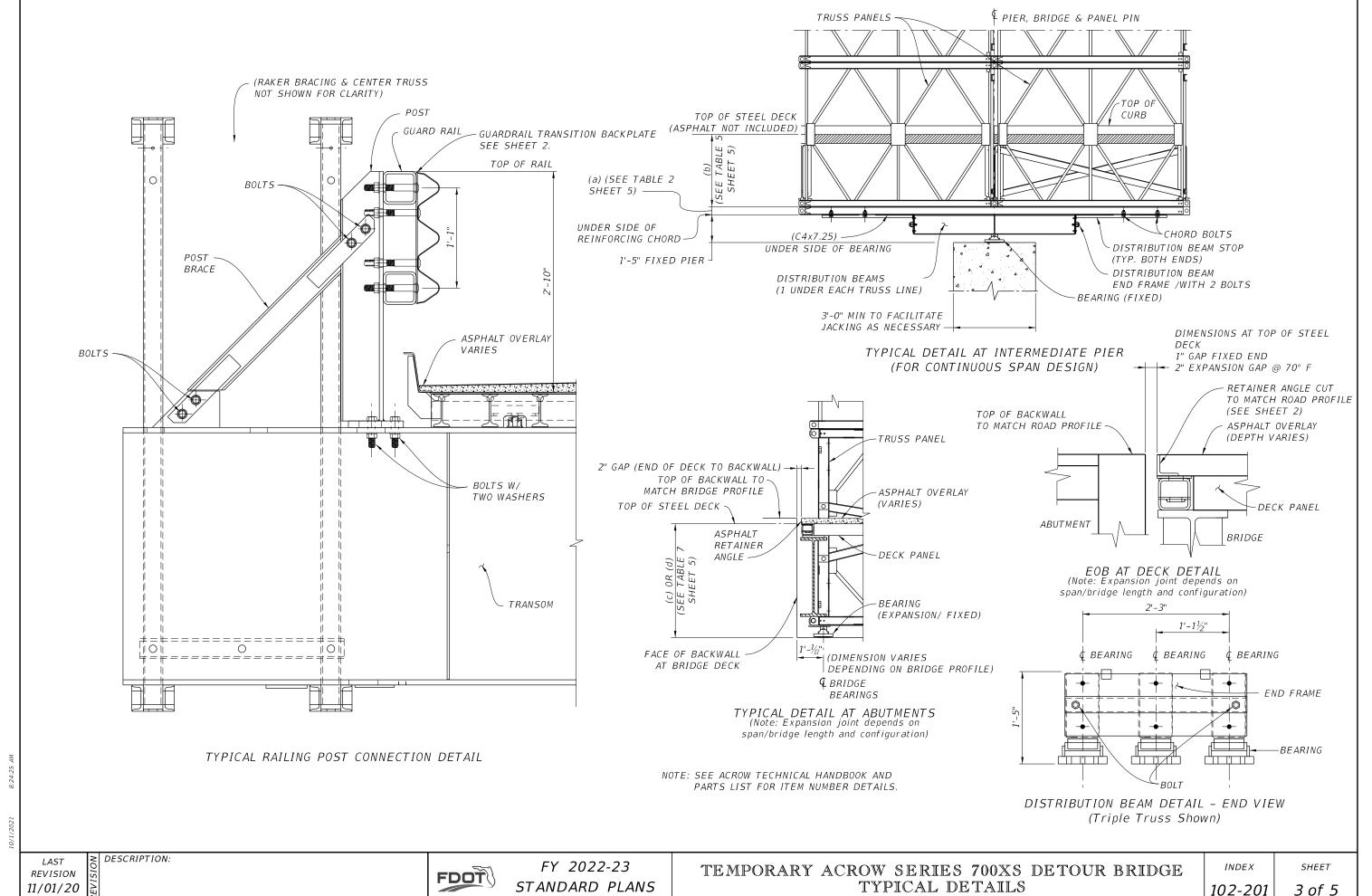
NOTE: SEE ACROW TECHNICAL HANDBOOK AND PARTS LIST FOR ITEM NUMBER DETAILS

**REVISION** 11/01/20

DESCRIPTION:

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FY 2022-23 STANDARD PLANS



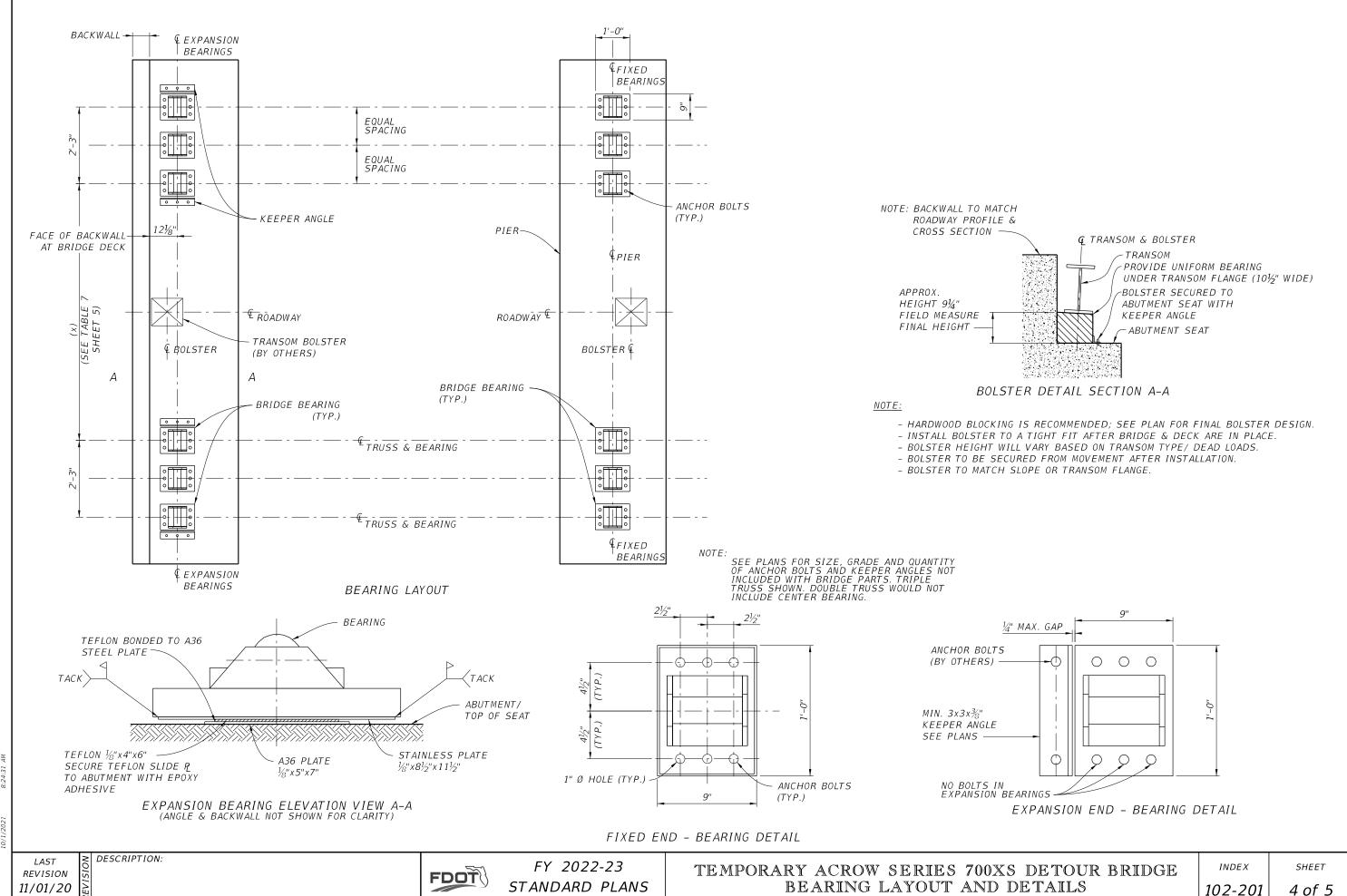


TABLE 1				
	Swaybrace / Transom Brace			
Roadway Transom Part # Part # Brace		Transom Brace Part #		
24	SC0017	AB590	AB515	AB519
36	AB957	AB891	AB891	AB519
42	AB978	AB979	AB979	AB519

TABLE 2					
(a) Reinfo	(a) Reinforcing Chord Thickness				
Regular Reinforcing Chord Thickness	Heavy Reinforcing Chord Thickness	SuperHeavy Reinforcing Chord Thickness			
4"	5"	6"			

Bridge Roadway width (ft)	Transom Part #	of Truss Chord to top of Transom
24	SC0017	285⁄ <sub>16</sub> "
36	AB957	40¾6"
42	AB978	43"

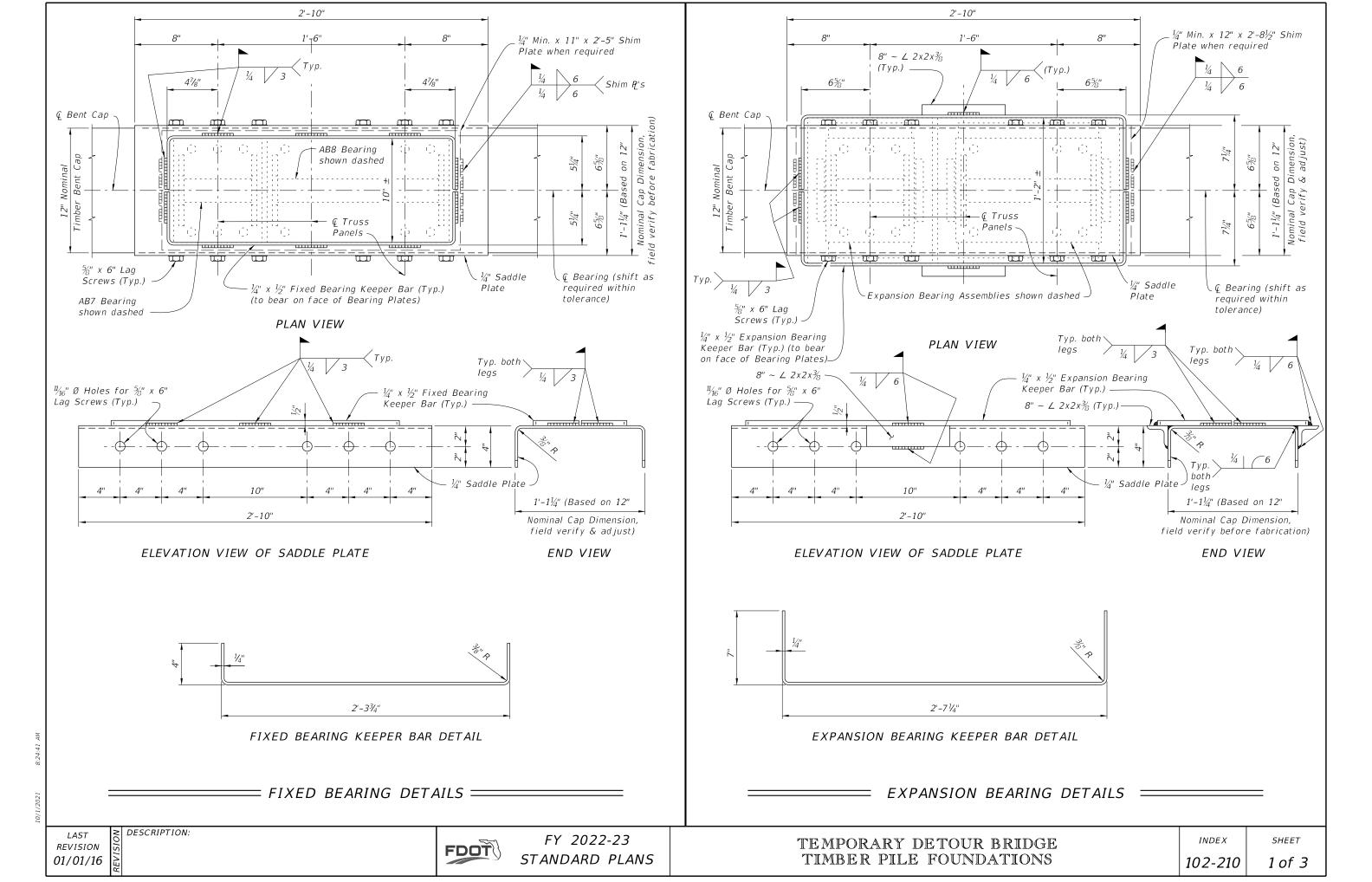
TABLE 3

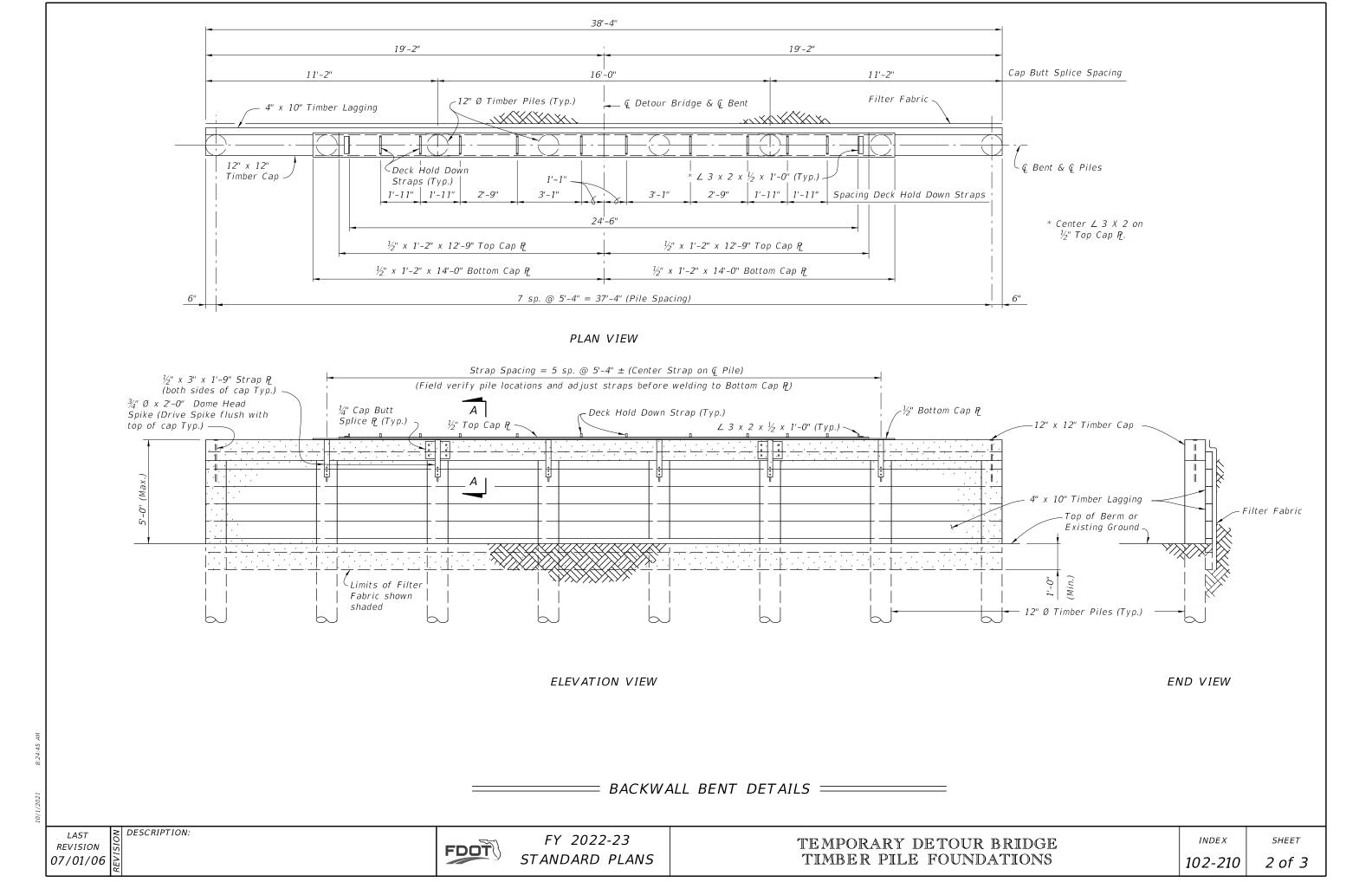
TABLE 4				
Bridge Roadway width (ft)	Transom Part #	(x) • to inner truss to • truss	(y) Transom Beam Length	
24	SC0017	26'-1"	31'-4"	
36	AB957	38'-4 <sup>13</sup> / <sub>16</sub> "	43'-7 <sup>13</sup> / <sub>16</sub> "	
42	AB978	44'-43%"	49'-7 <del>%</del> "	

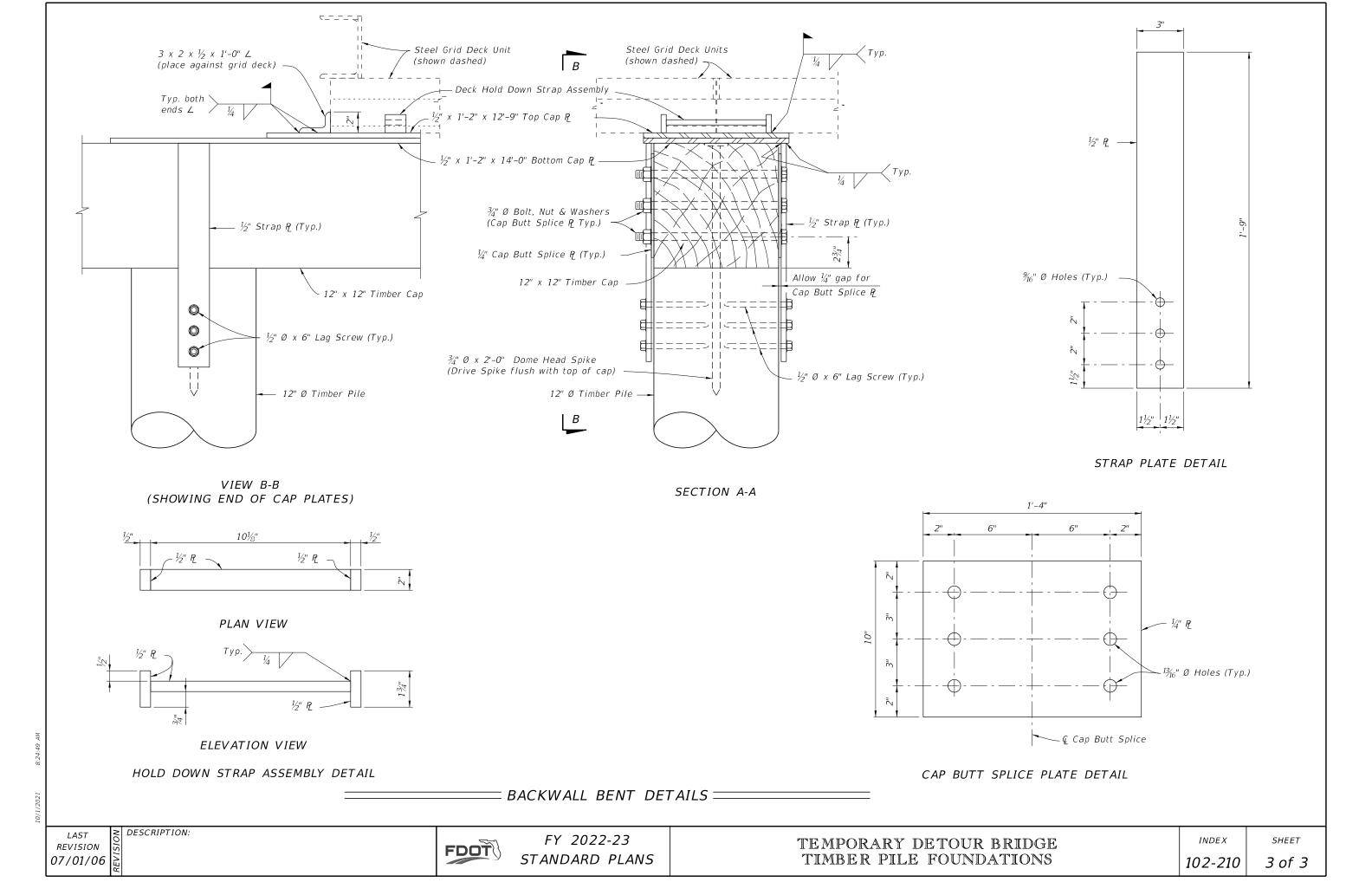
TABLE 5				
Bridge Roadway width (ft)	Transom Part #	(b) Height Bottom of Truss Chord to top of Deck		
24	SC0017	33 <sup>11</sup> / <sub>16</sub> "		
36	AB957	45% <sub>6</sub> "		
42	AB978	48 <del>%</del> "		

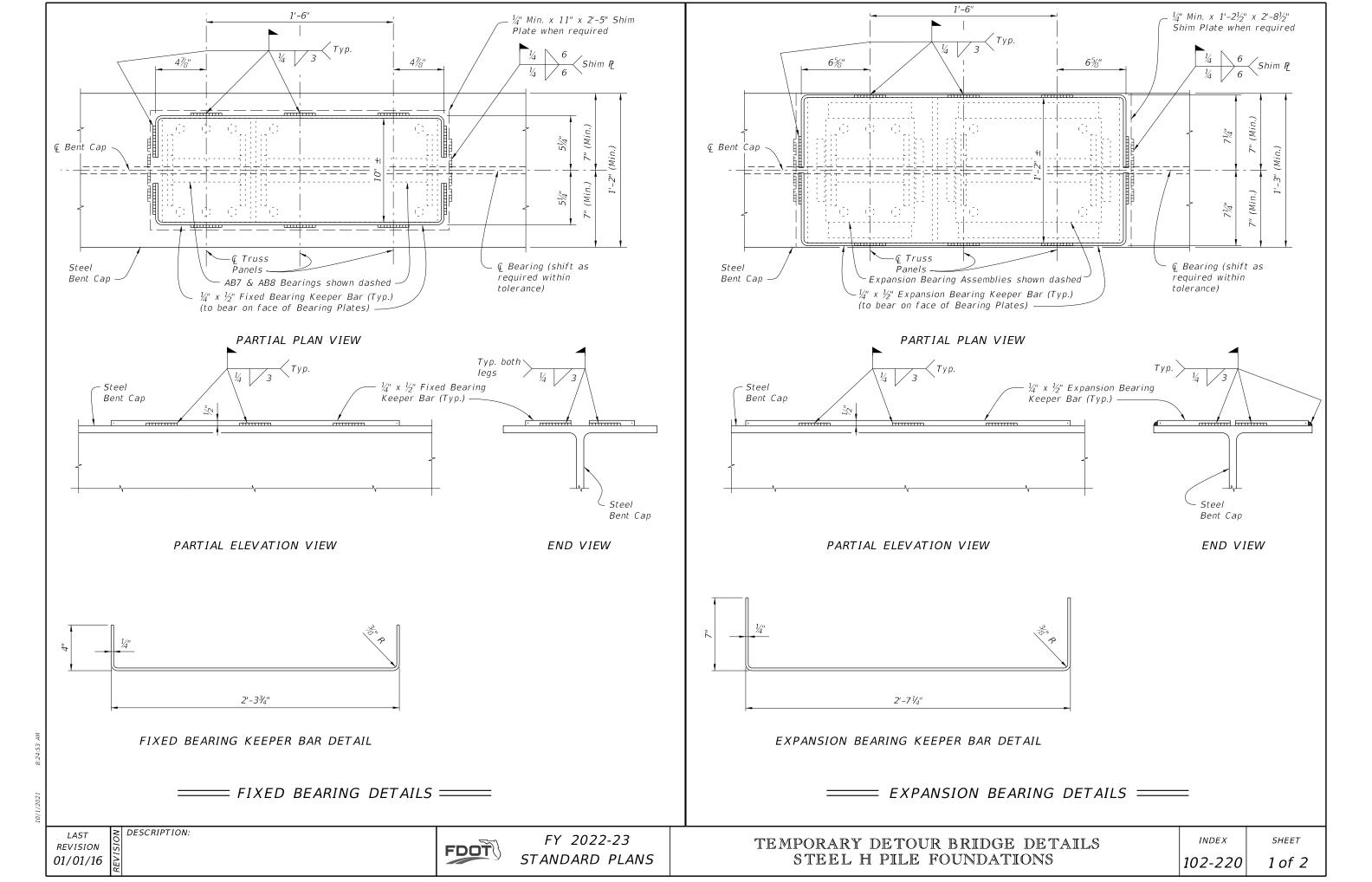
TABLE 6					
Bridge Roadway width (ft)	Transom Part #	(c) Height Bottom of fixed bearing to top of Deck	(d) Height Bottom of expansion bearing to top of Deck		
24	SC0017	39½"	39¾ <sub>6</sub> "		
36	AB957	50 <sup>15</sup> ⁄16"	515⁄ <sub>16</sub> "		
42	AB978	53¾"	54½"		

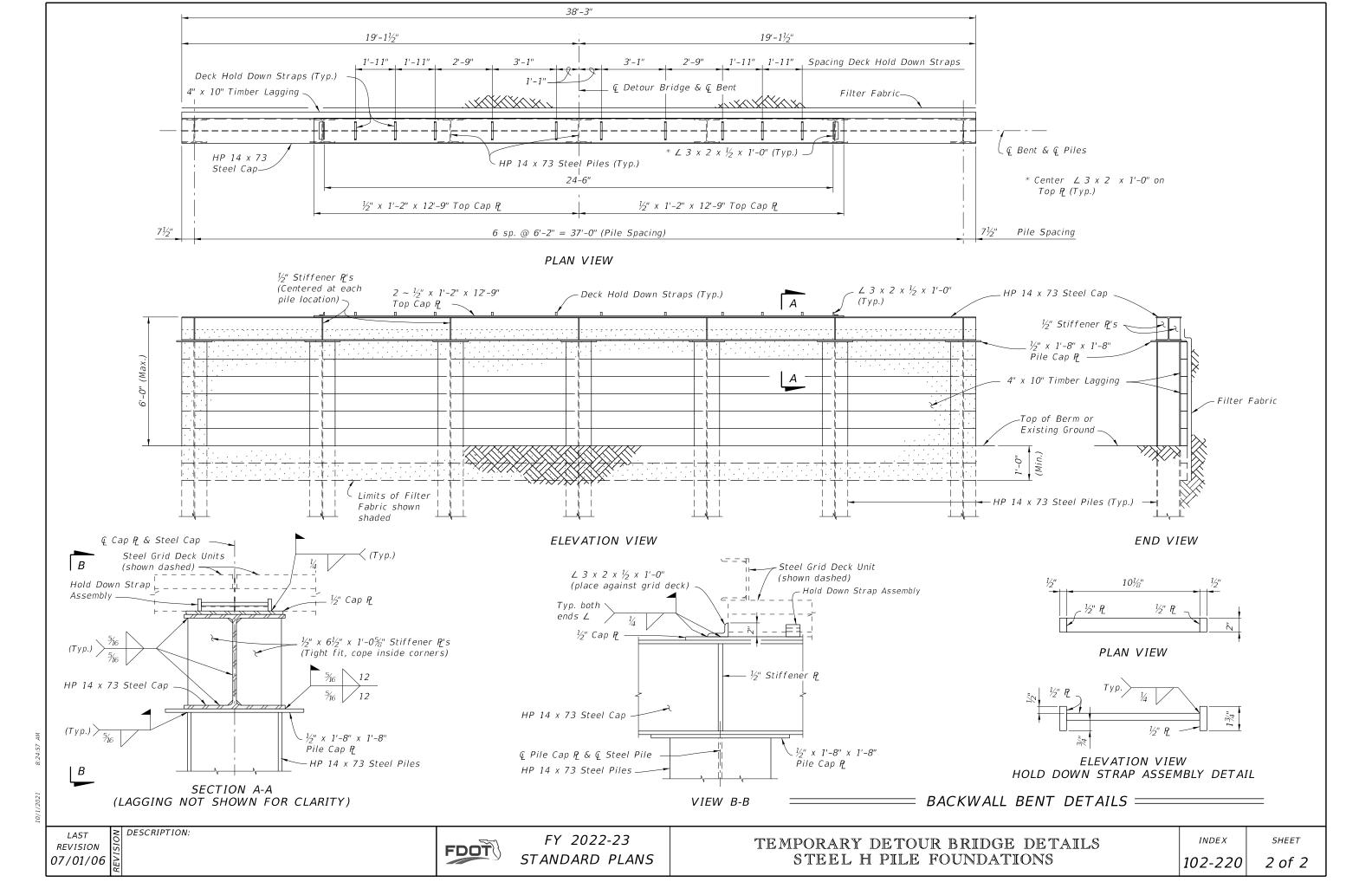
TABLE 7				
Bridge Roadway width (ft)	Transom Part #	(x) • to inner truss to • tinner truss		
24	SC0017	26'-1"		
36	AB957	38'-4 <sup>13</sup> ⁄16"		
42	AB978	44'-43/8"		

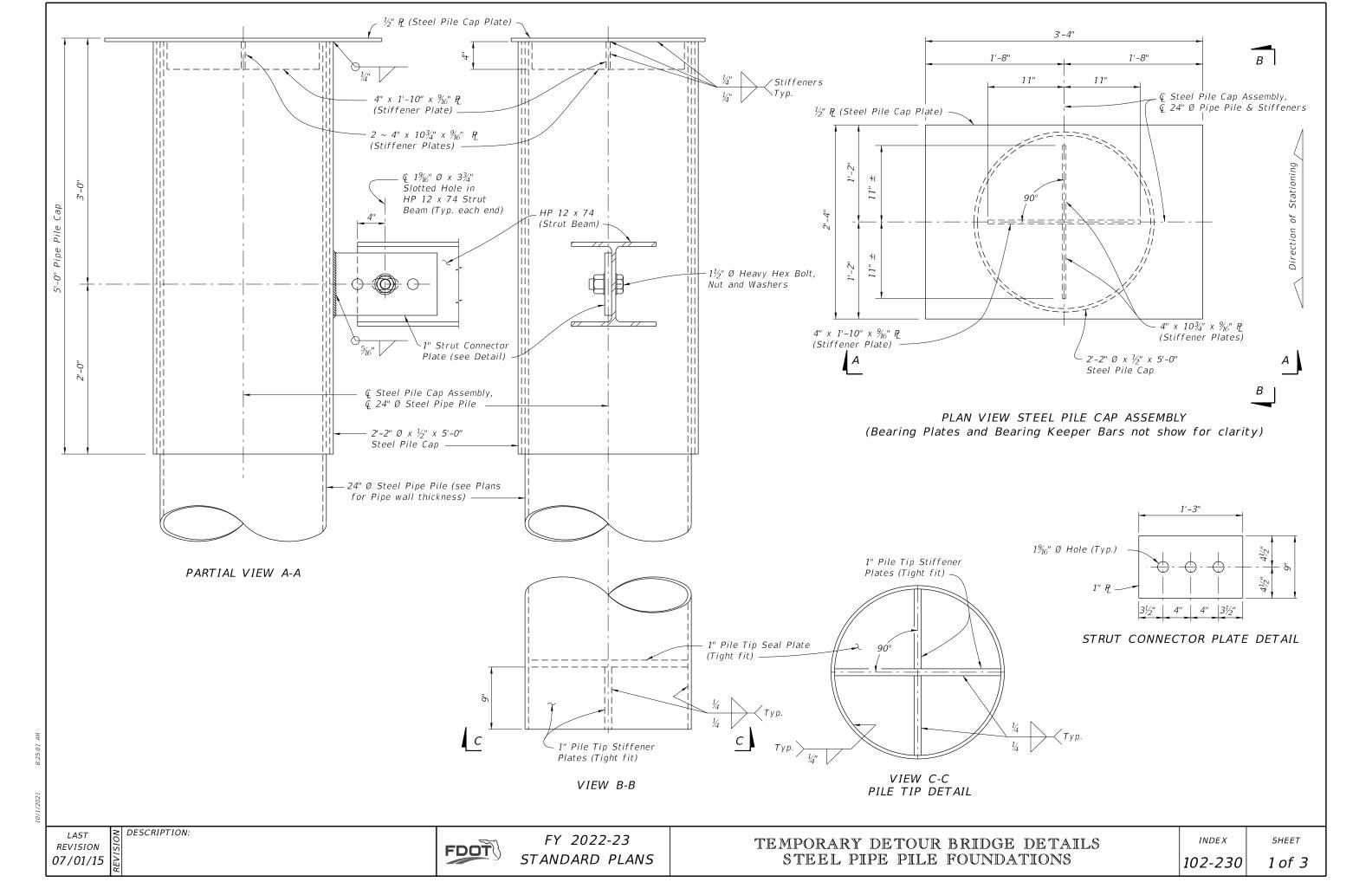


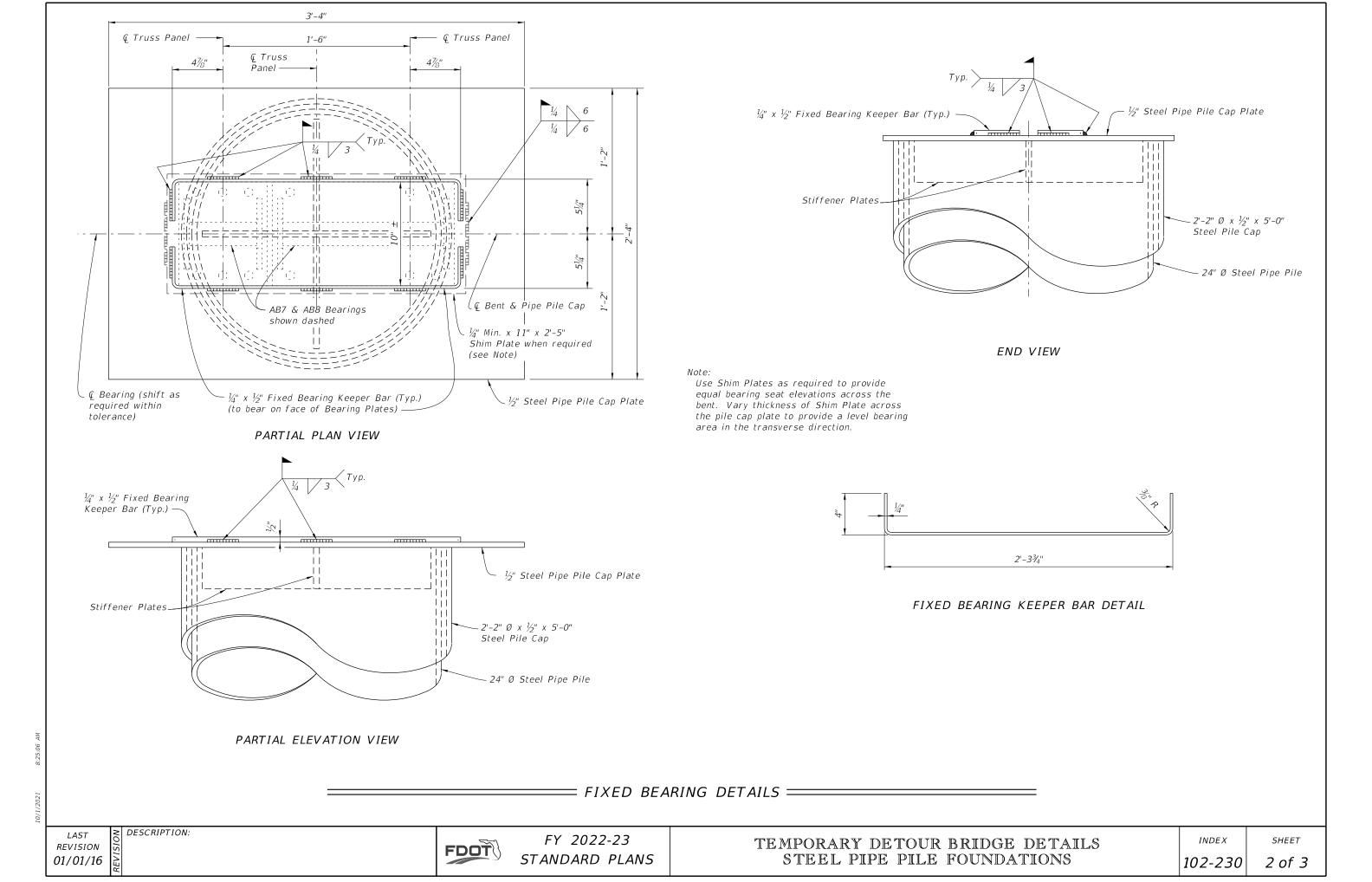


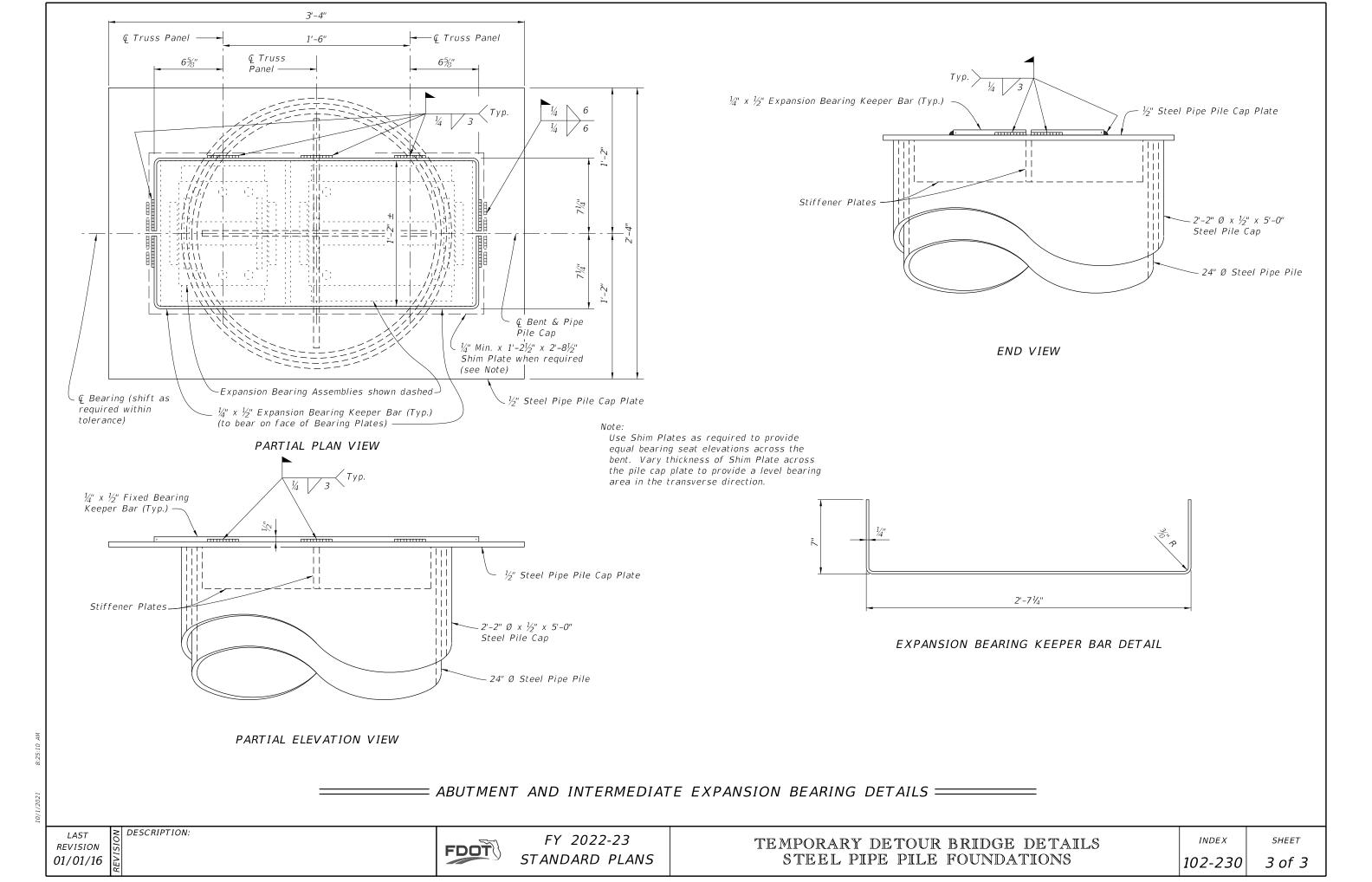


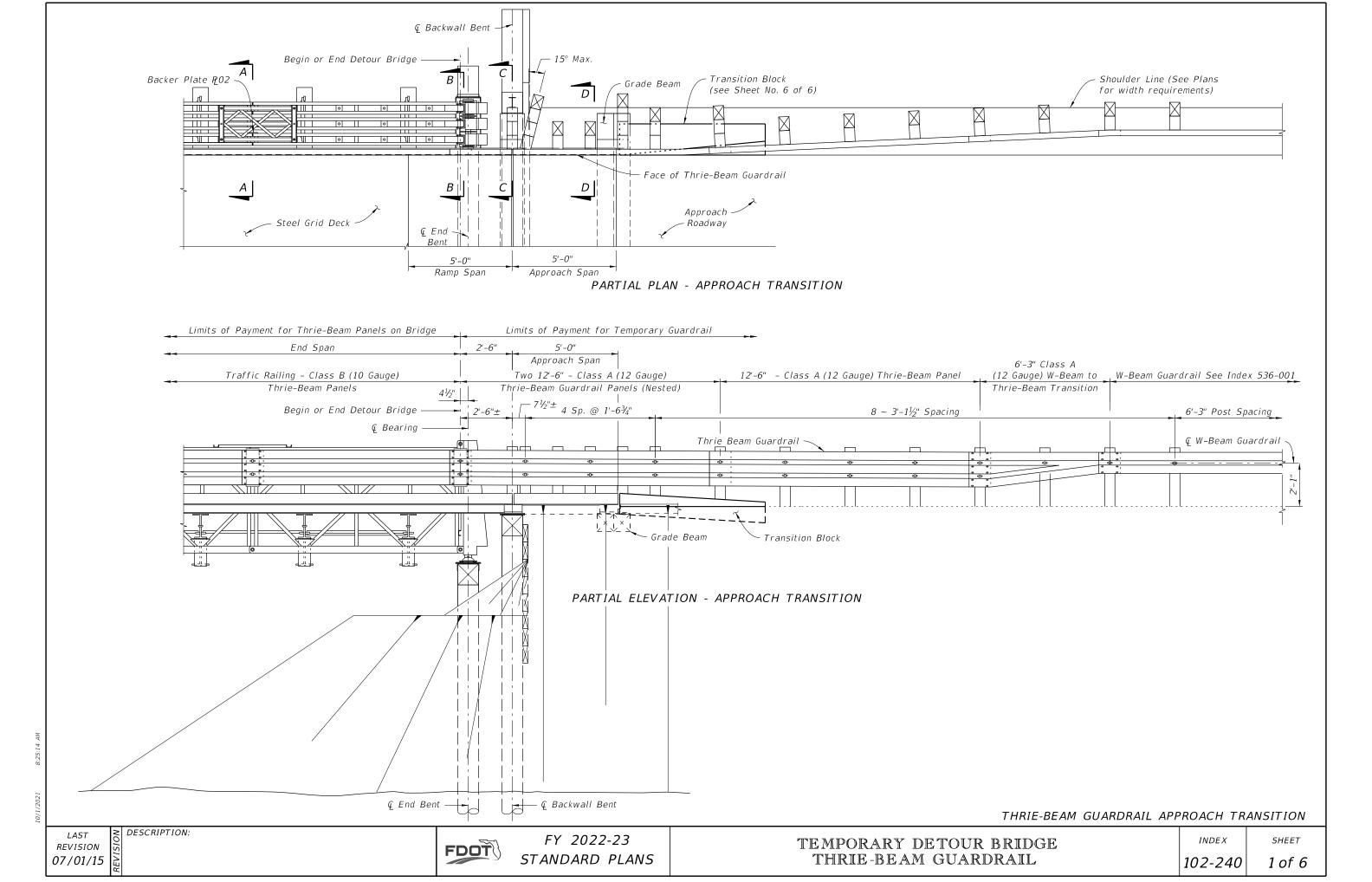


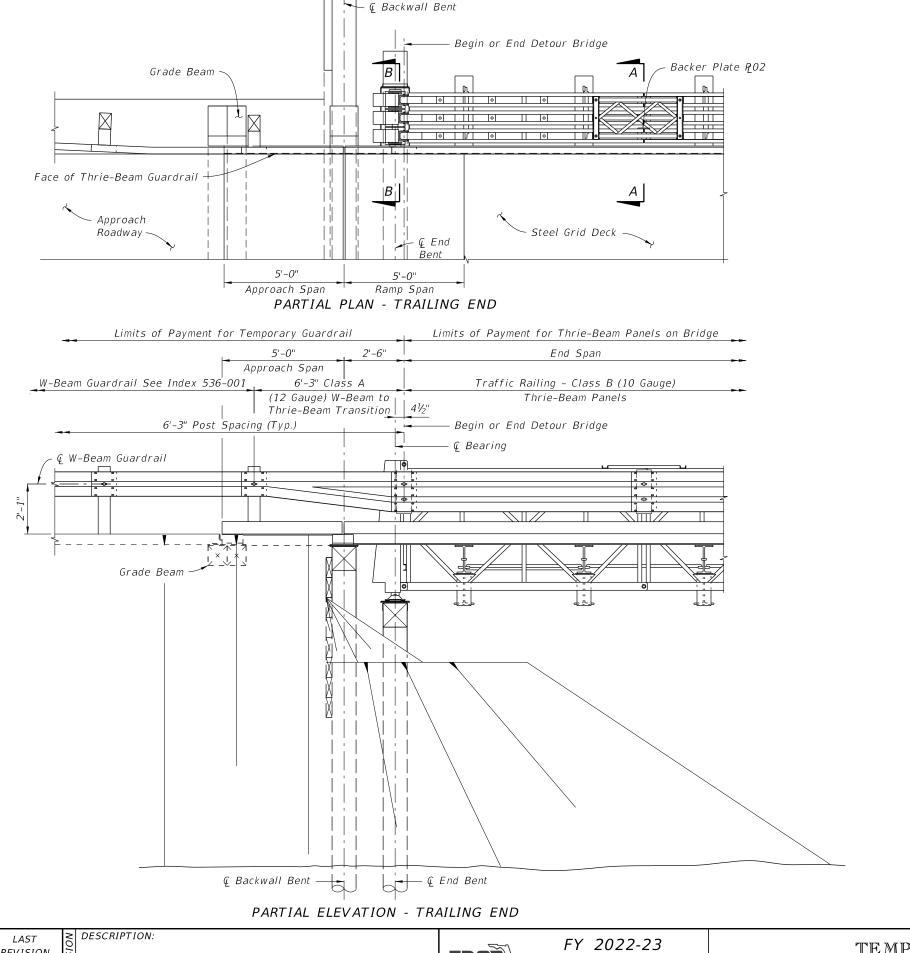


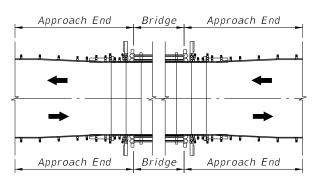




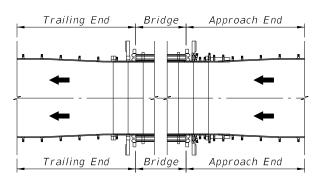








TWO-WAY TRAFFIC



ONE-WAY TRAFFIC END TRANSITION APPLICATION DETAILS

THRIE-BEAM GUARDRAIL TRAILING END TRANSITION

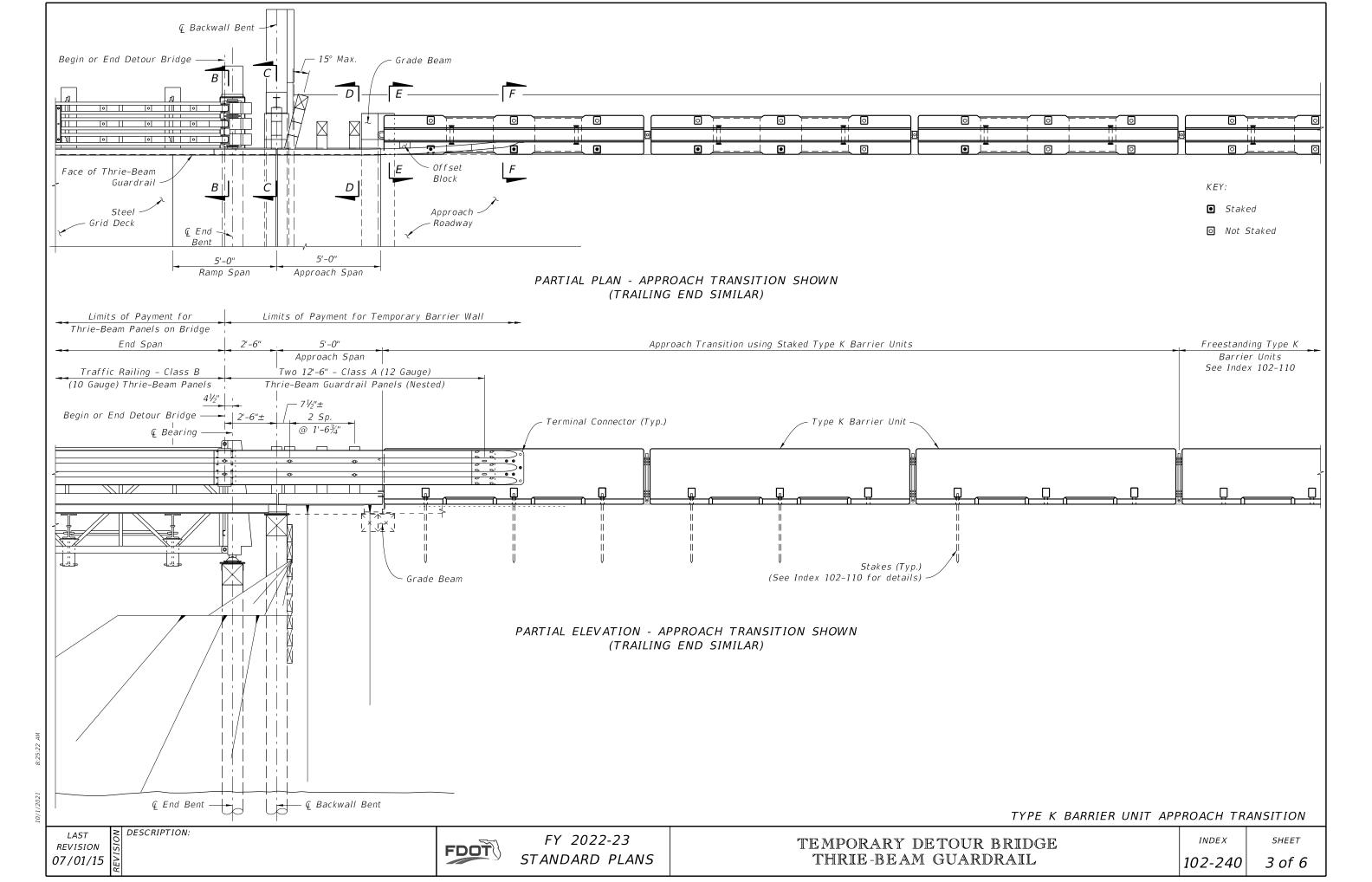
REVISION 07/01/15

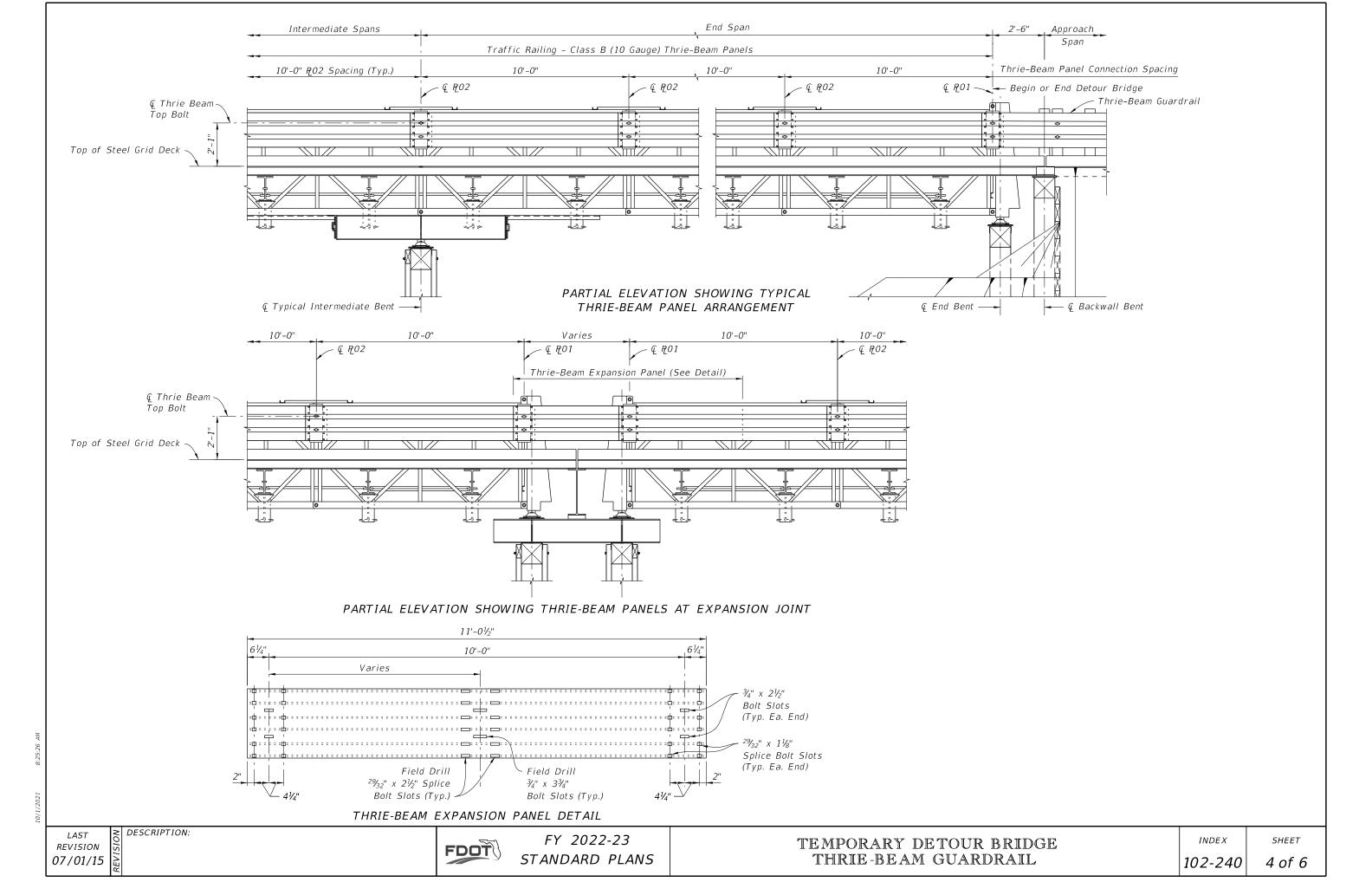
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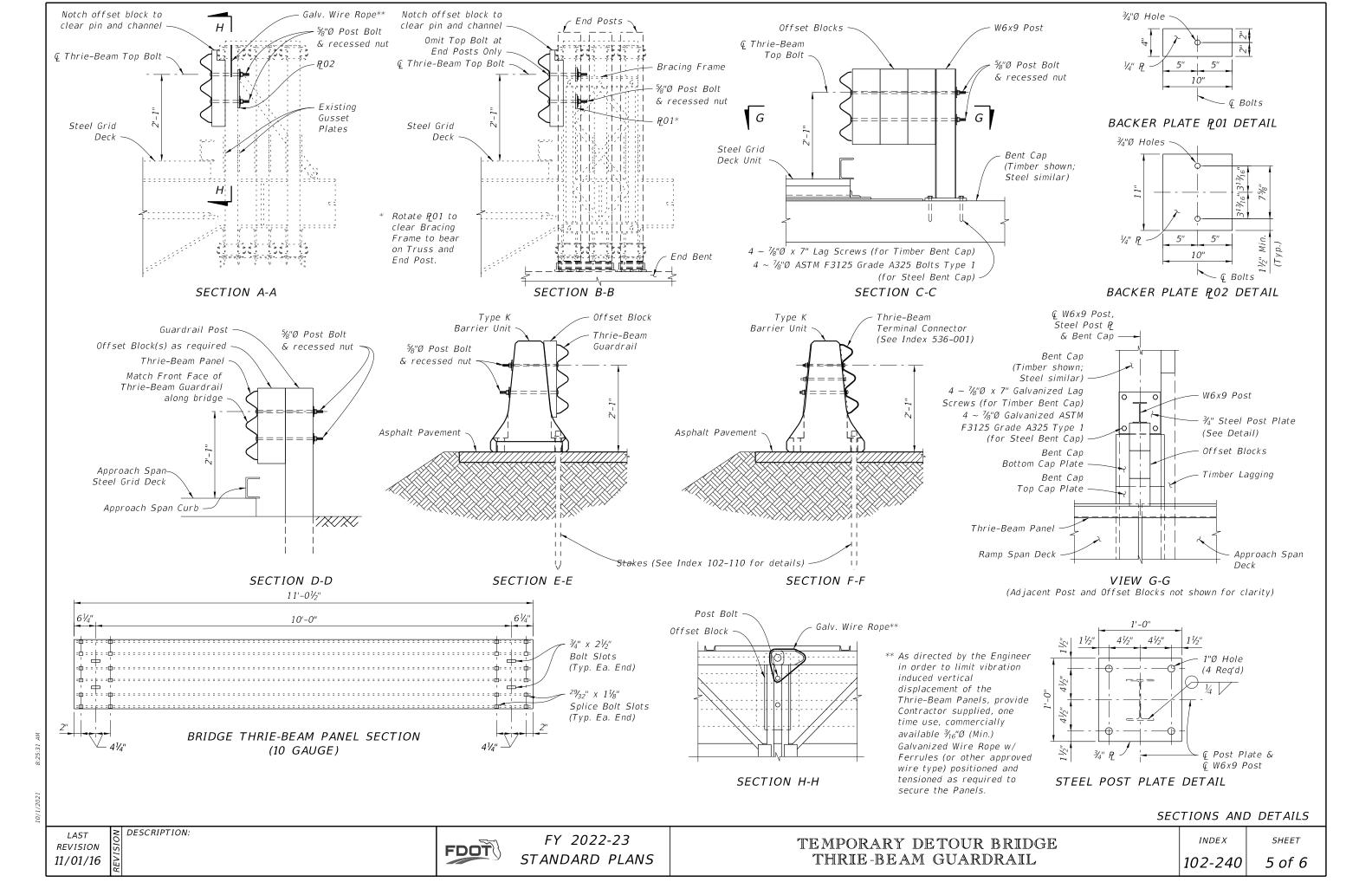
STANDARD PLANS

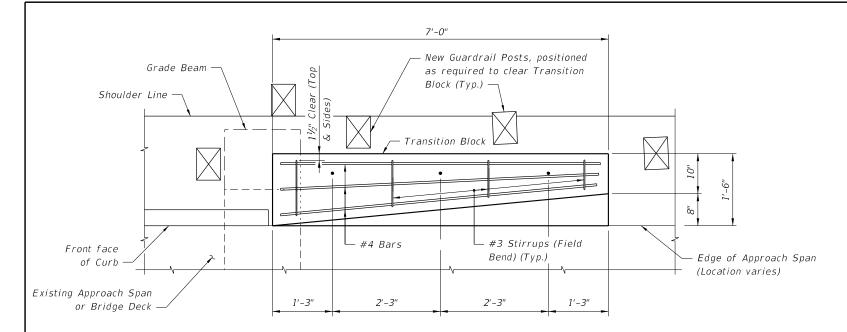
TEMPORARY DETOUR BRIDGE THRIE-BEAM GUARDRAIL

INDEX 102-240 SHEET

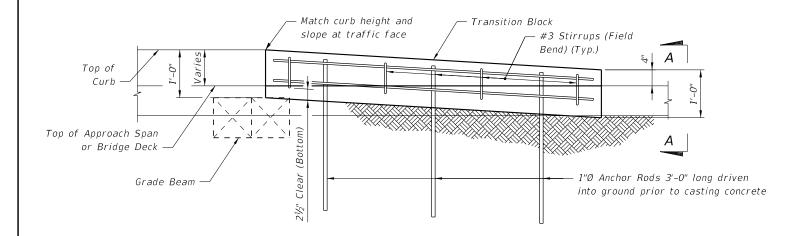






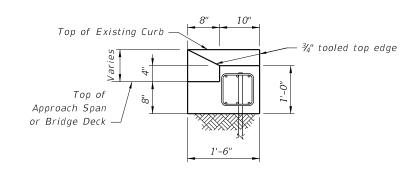


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

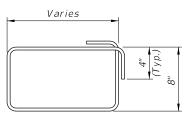


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUA	ANTITIE	S
ITEM	UNIT	QUANTITY
Concrete Class NS	CY	0.4
Reinforcing Steel	LB	61
Guardrail (Reset)	LF	12.5



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

REVISION 07/01/13

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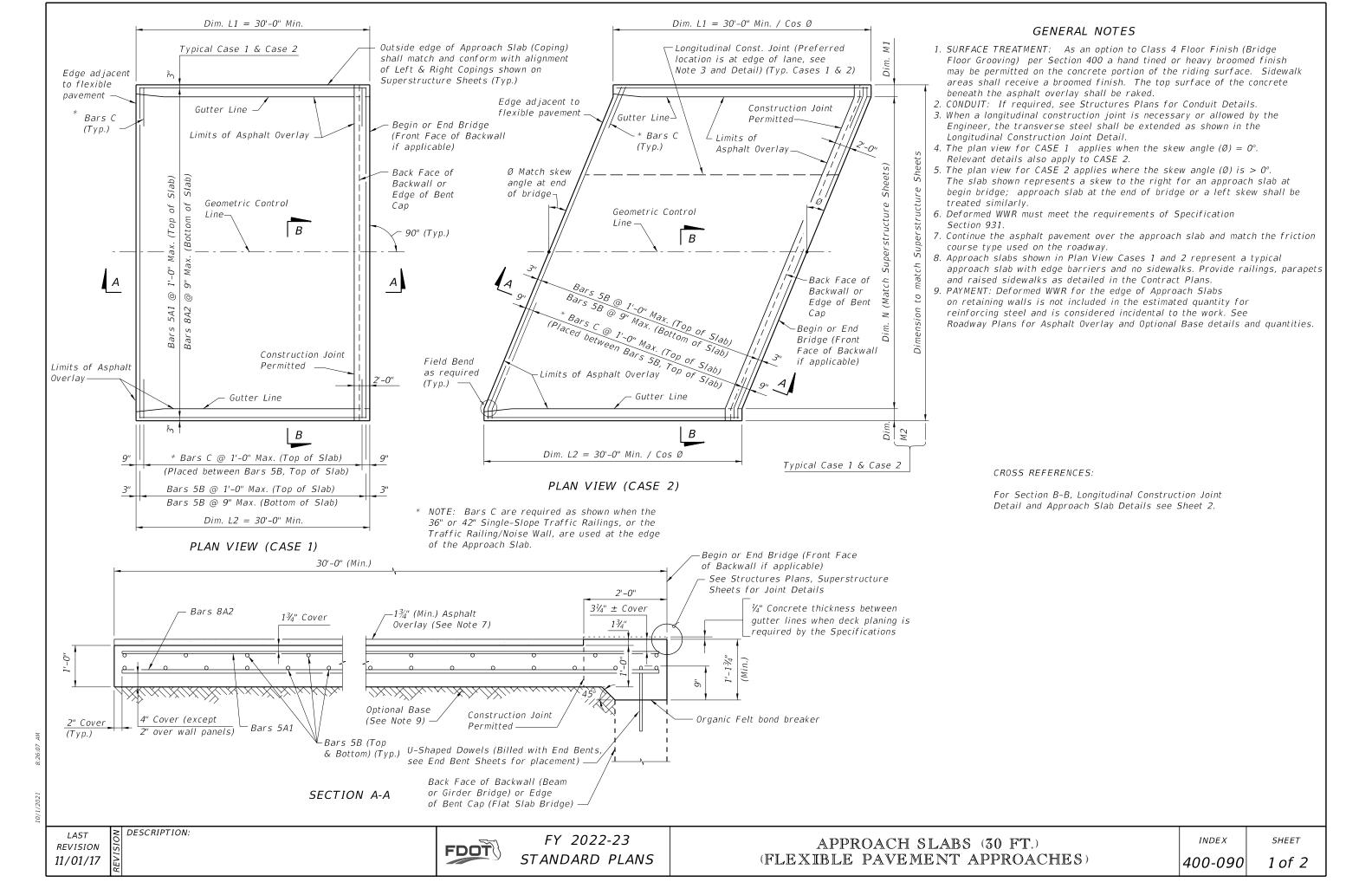
FY 2022-23 STANDARD PLANS

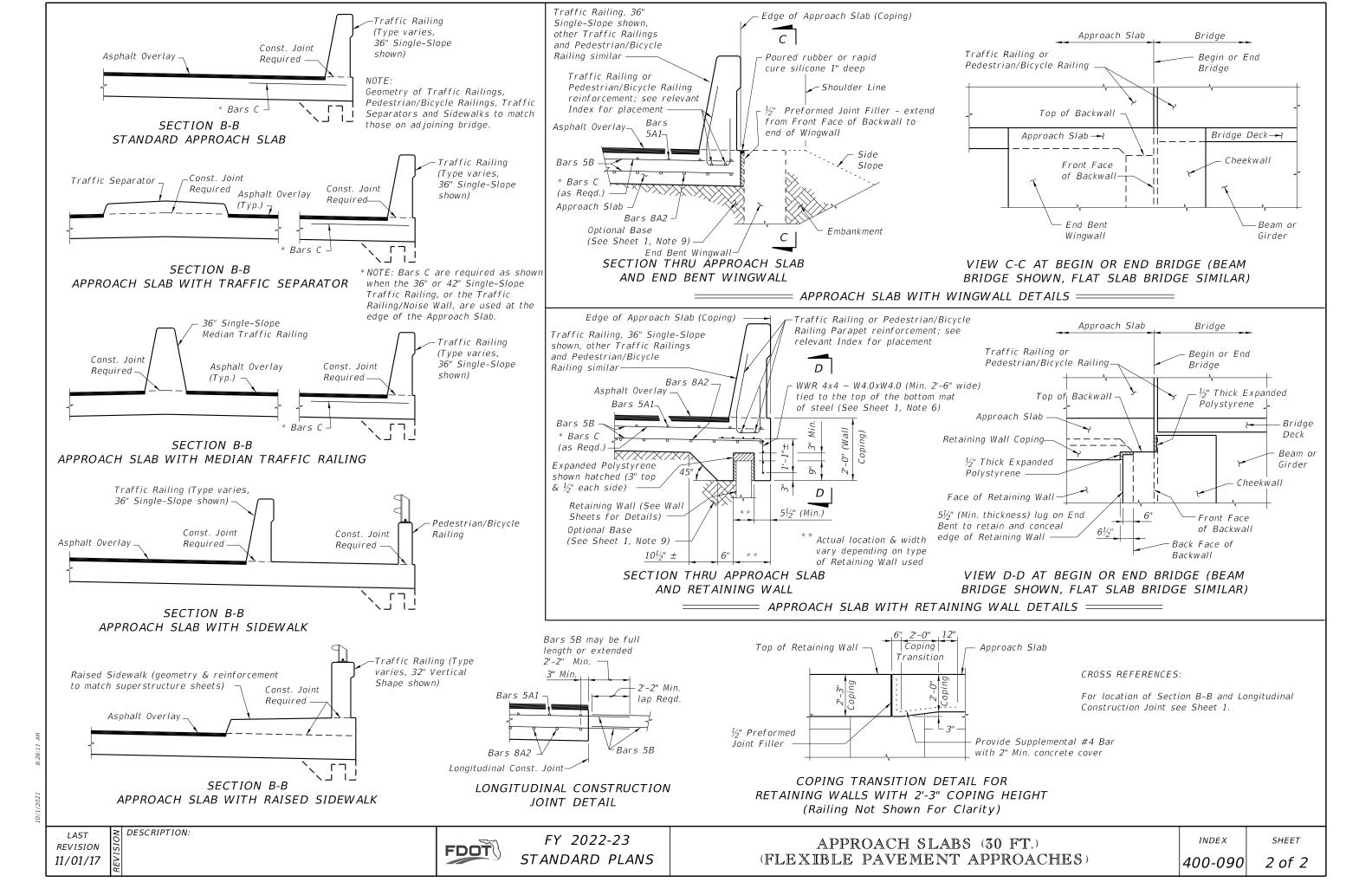
TEMPORARY DETOUR BRIDGE

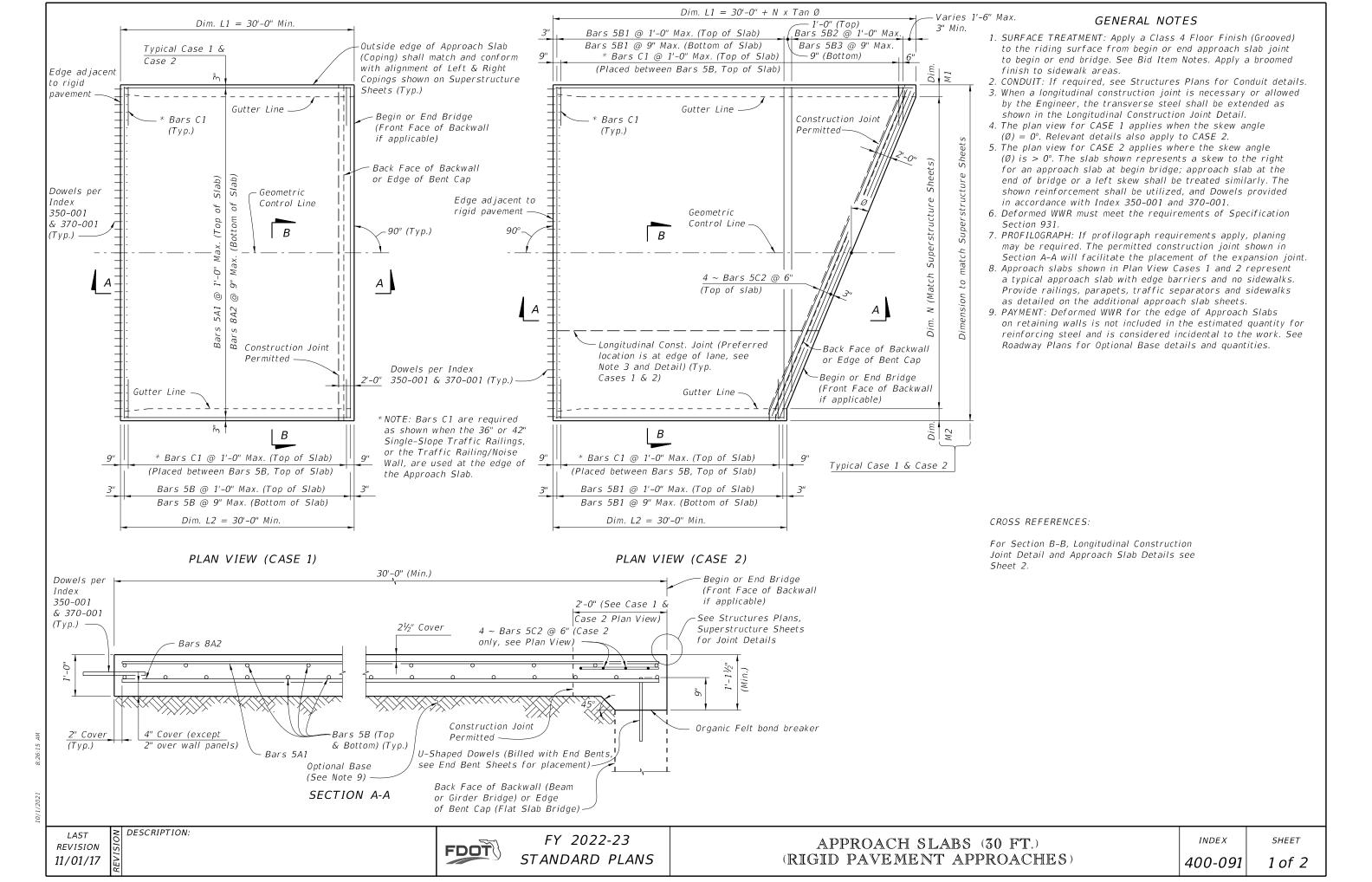
INDEX

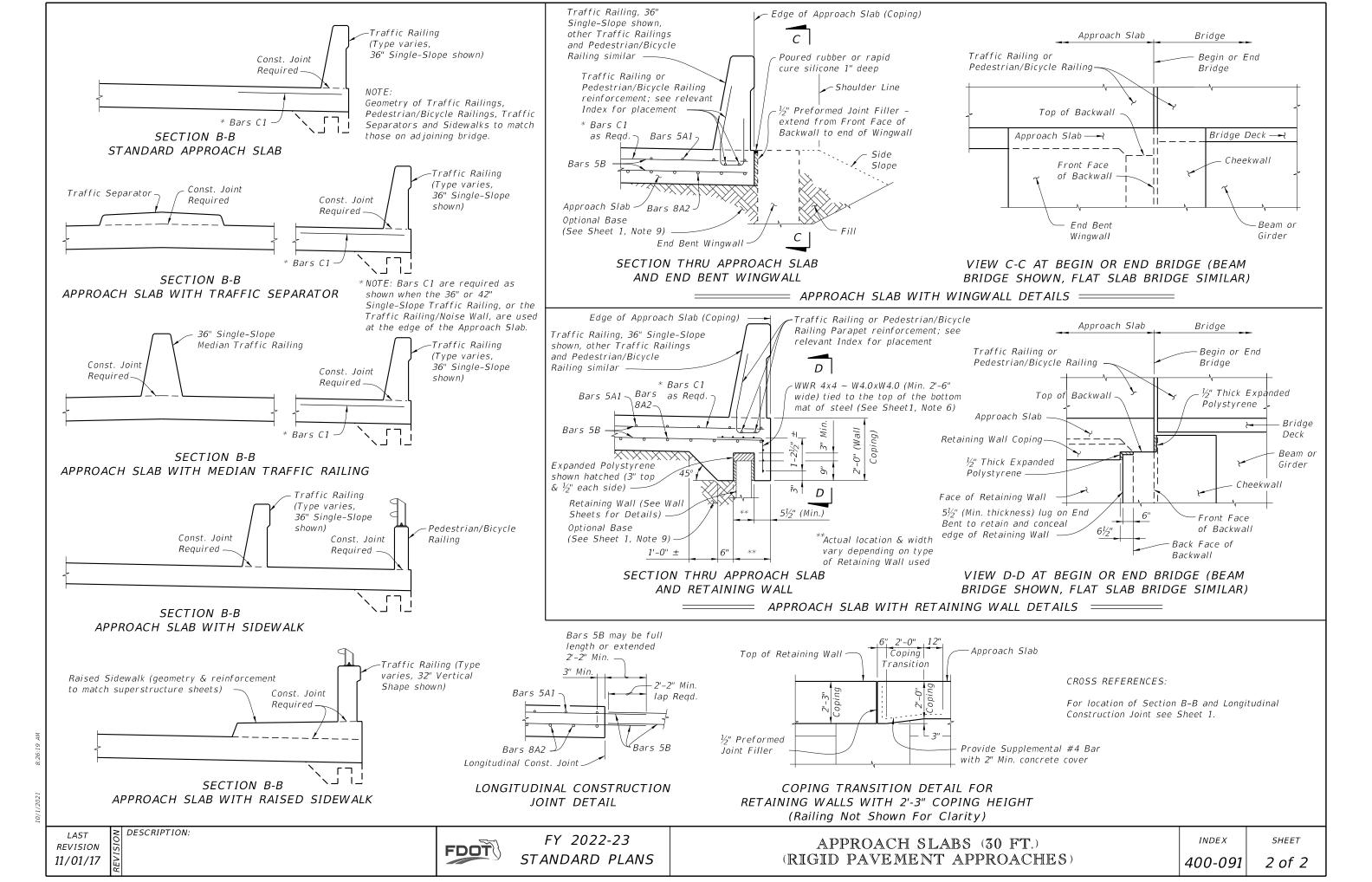
SHEET

102-240 6 of 6

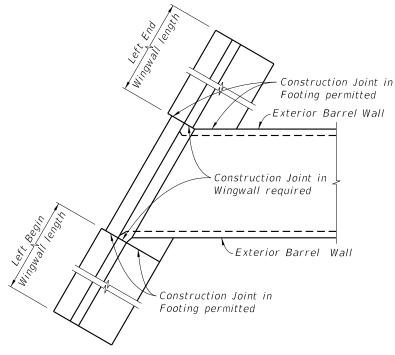








NOTE: All headwall and culvert skew angles are measured in degrees from a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic "B".



PART PLAN SHOWING PARALLEL WINGWALLS AND LOCATION OF CONSTRUCTION JOINTS

NOTE:

DESCRIPTION:

Construction Joints in wingwalls and footings are located as follows: For non-skewed wingwalls they are located adjacent to the exterior face of the exterior barrel wall; when the Ç of wingwall and Ç of exterior barrel wall results in an acute angle see Left End Wingwall above, and when the angle is obtuse see Left Begin Wingwall above and Detail C (Sheet 5).

## **GENERAL NOTES:**

LIVE LOAD: HL-93.

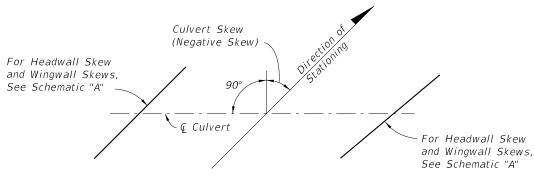
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 8.

REINFORCING STEEL: See the "Box Culvert Data Tables" in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.

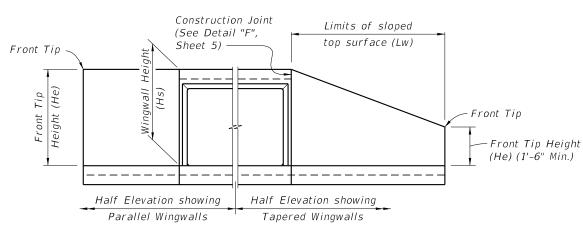


## SCHEMATIC "B" - PLAN VIEW CULVERT ALIGNMENT

NOTE: For Culvert Skew see Contract Plans.

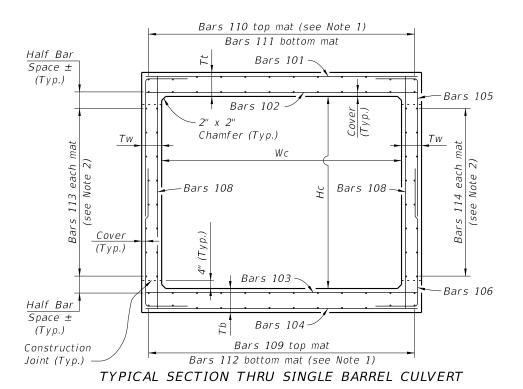
TAB	LE 1 - MI	NIMUM E	BAR SP	LICE LEN	GTHS
	FOR LON	IGITUDIN.	AL REI	NFORCIN	G
BAR	SPLICE (	CLASS B)	BAR	SPLICE (	CLASS B)
SIZE	CLASS II	CLASS IV	SIZE	CLASS II	CLASS IV
	(3400 psi)	(5500 psi)		(3400 psi)	(5500 psi)
#3	1'-4"	1'-0"	#8	3'-5"	2'-8"
#4	1'-9"	1'-4"	#9	4'-3"	3'-4"
#5	2'-2"	1'-8"			
#6	2'-7"	2'-0"			
#7	3'-0"	2'-4"			

TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.



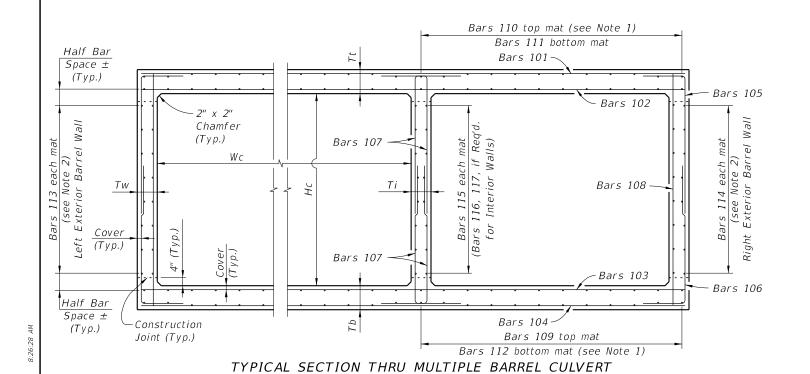
**END ELEVATION** OF CULVERT

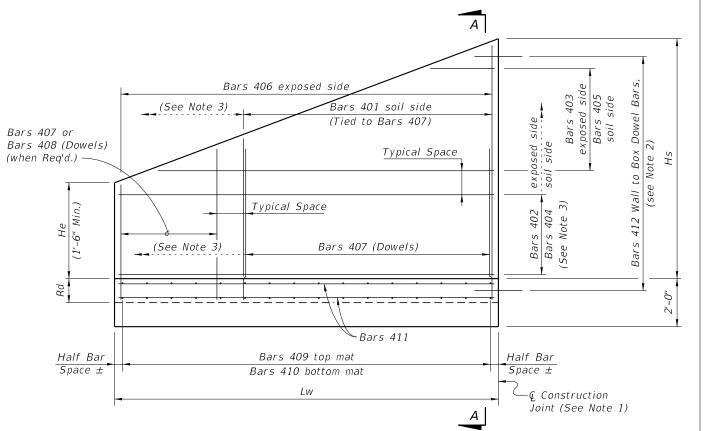
REVISION 11/01/16



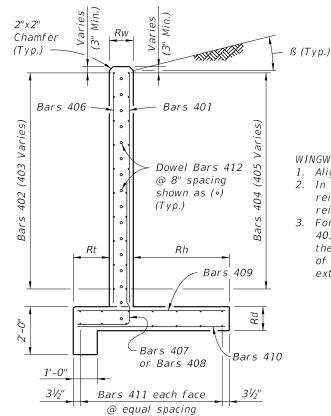
#### CULVERT BARREL NOTES:

- 1. Space Bars 110 and 112 with a bar in each corner, and at the Q of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4" clear.





WINGWALL ELEVATION - Variable Height (Left End shown - other corners similar)



#### WINGWALL NOTES:

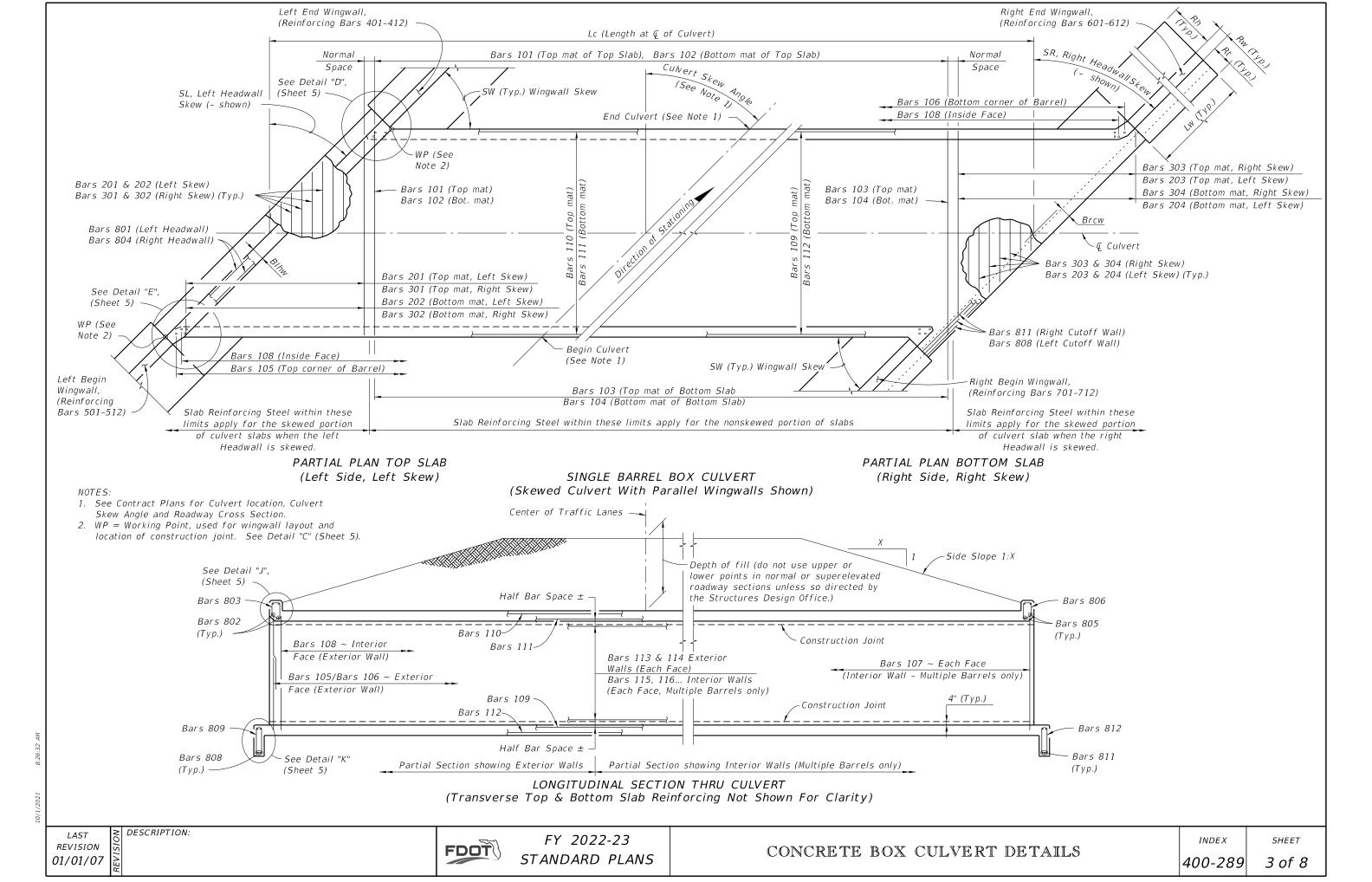
- 1. Align construction joint perpendicular to wingwall.
- 2. In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover.
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

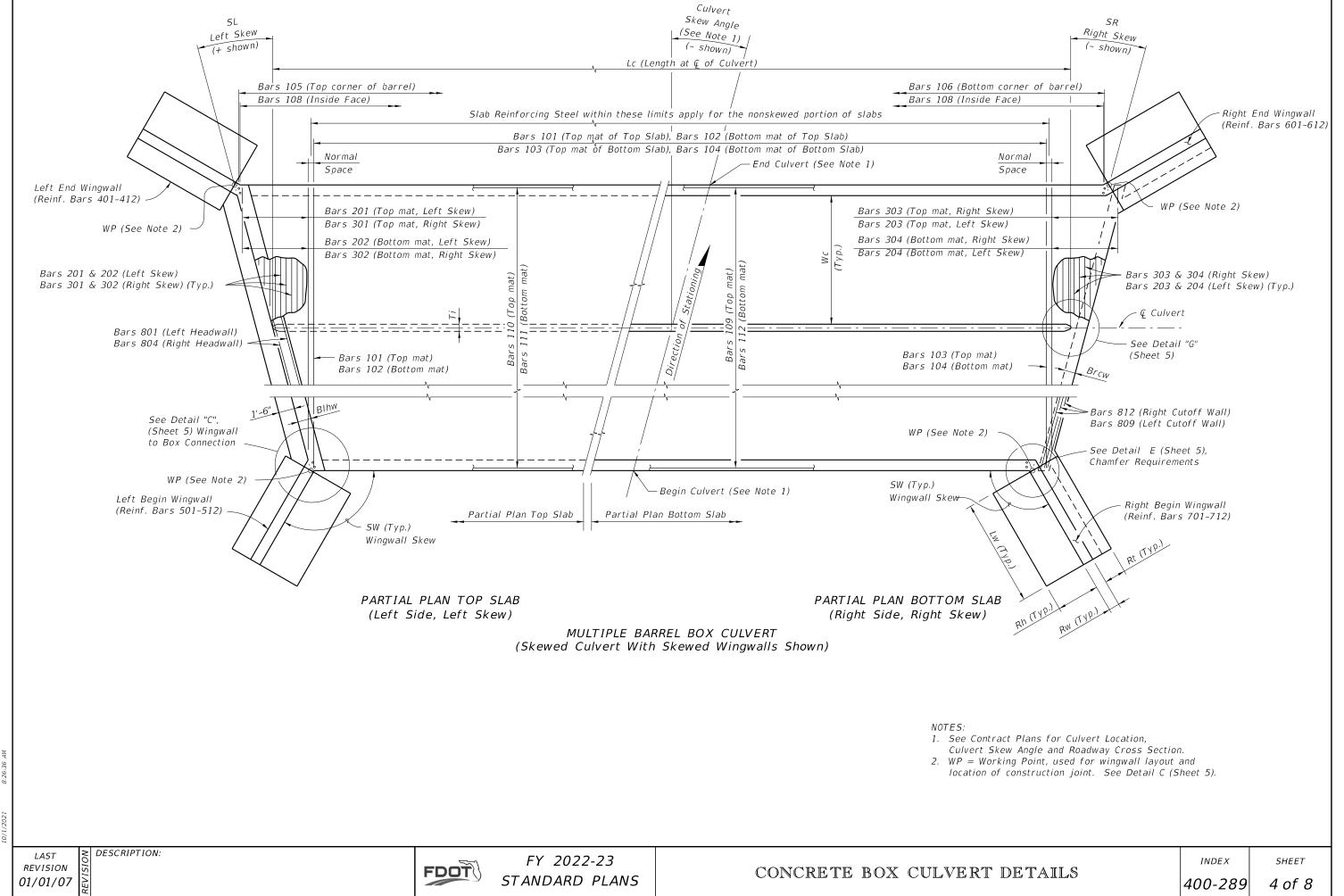
LAST O DESCRIPTION:
REVISION 5

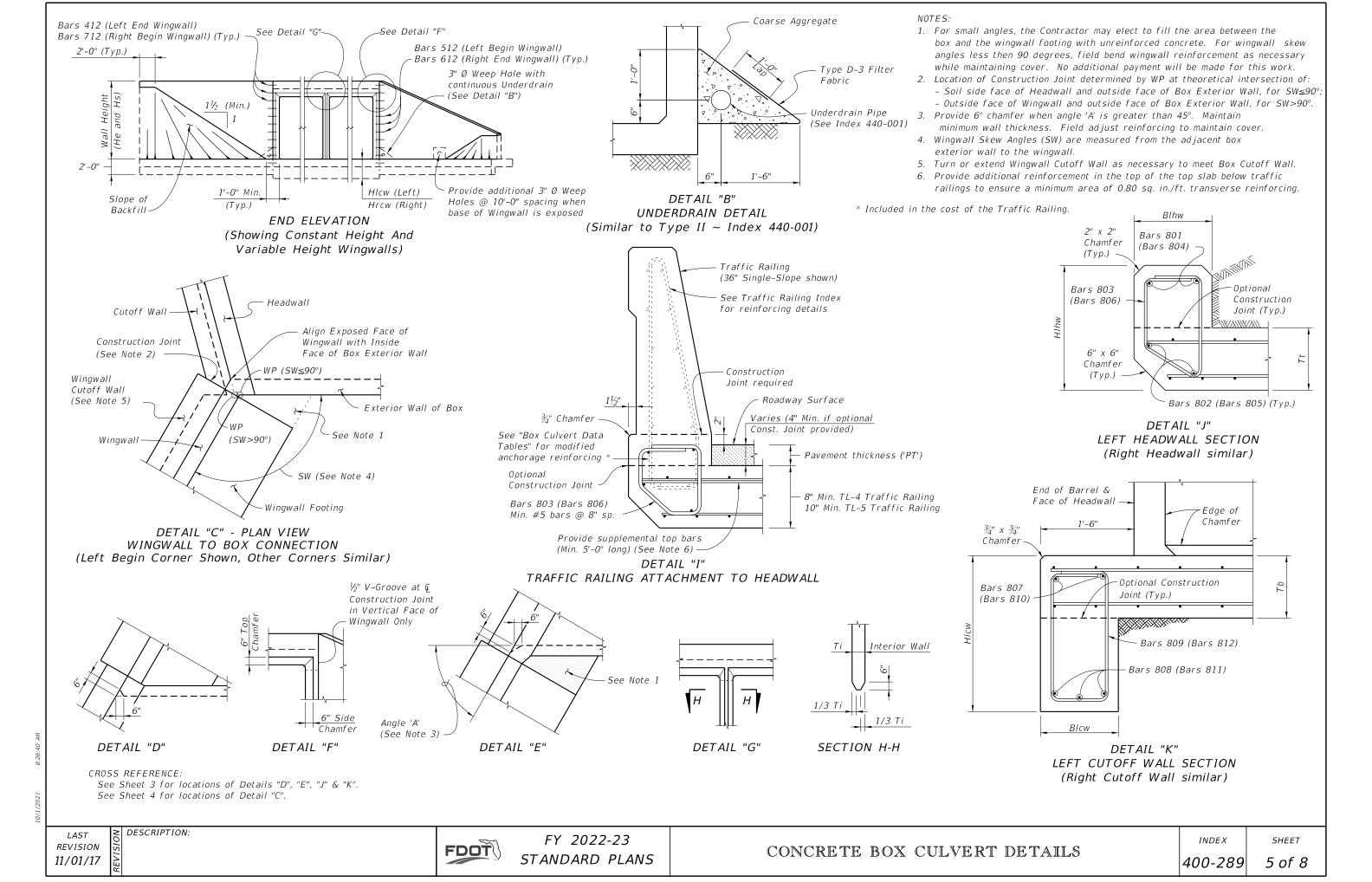
07/01/13

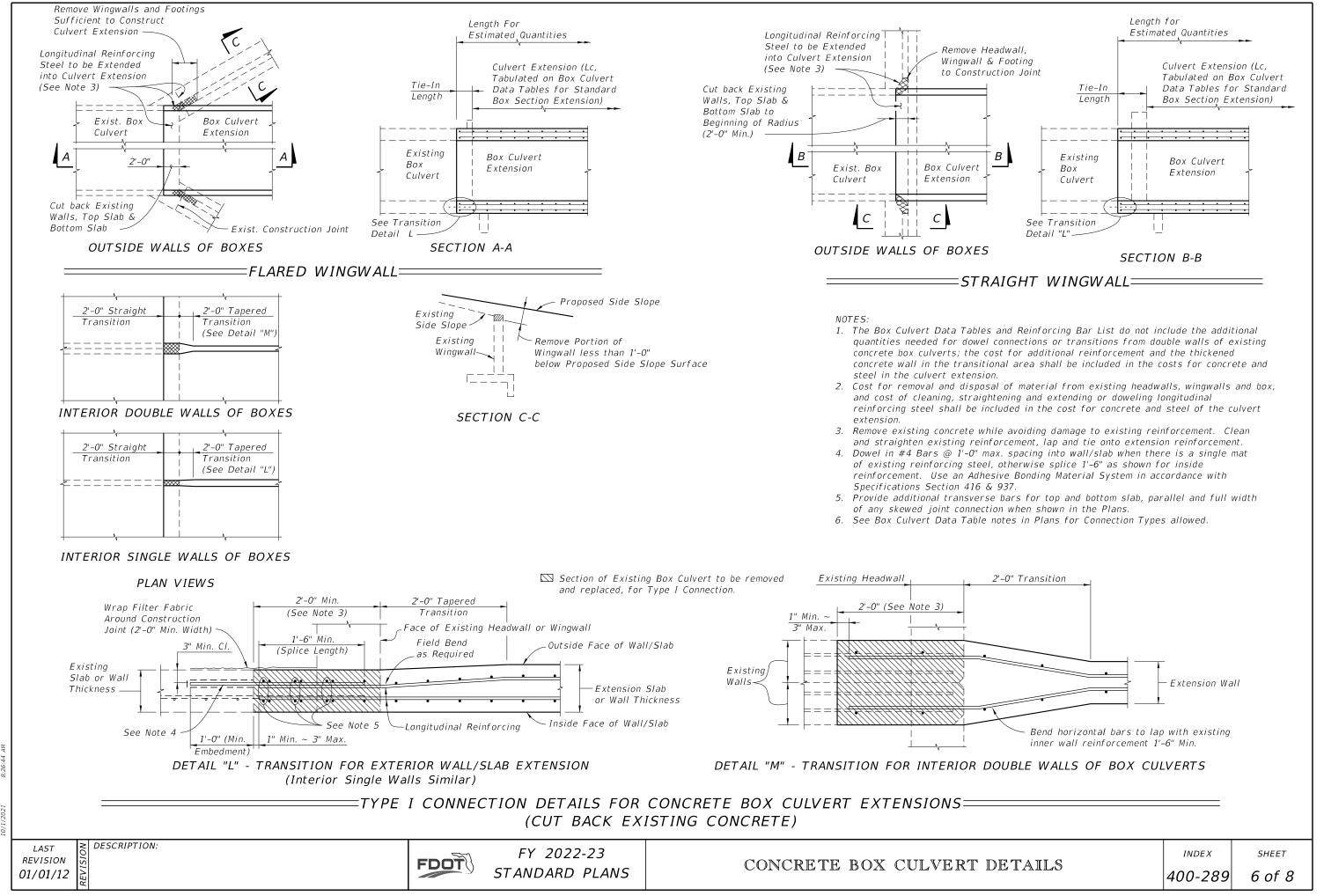
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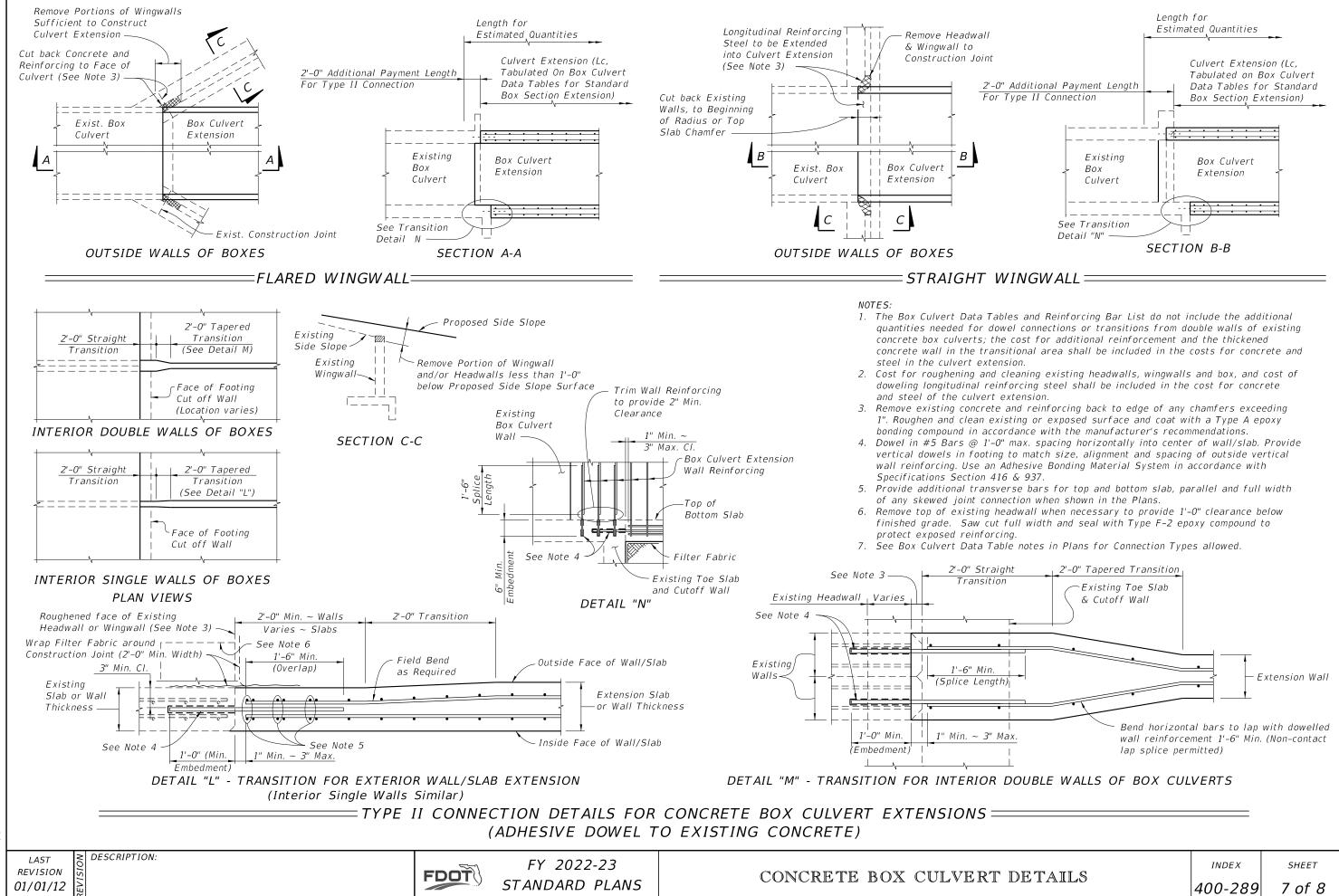
FY 2022-23 STANDARD PLANS WINGWALL SECTION A-A



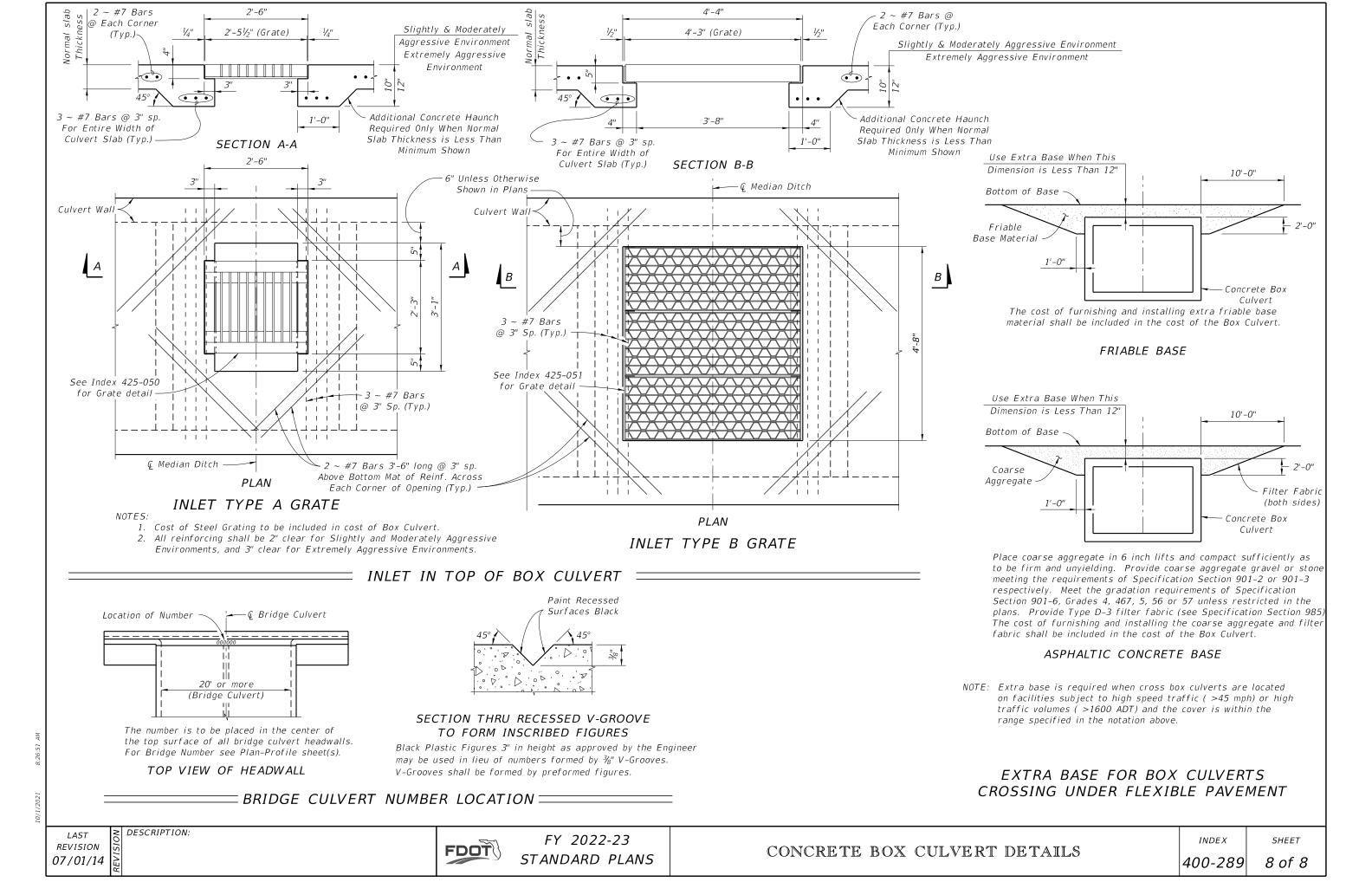


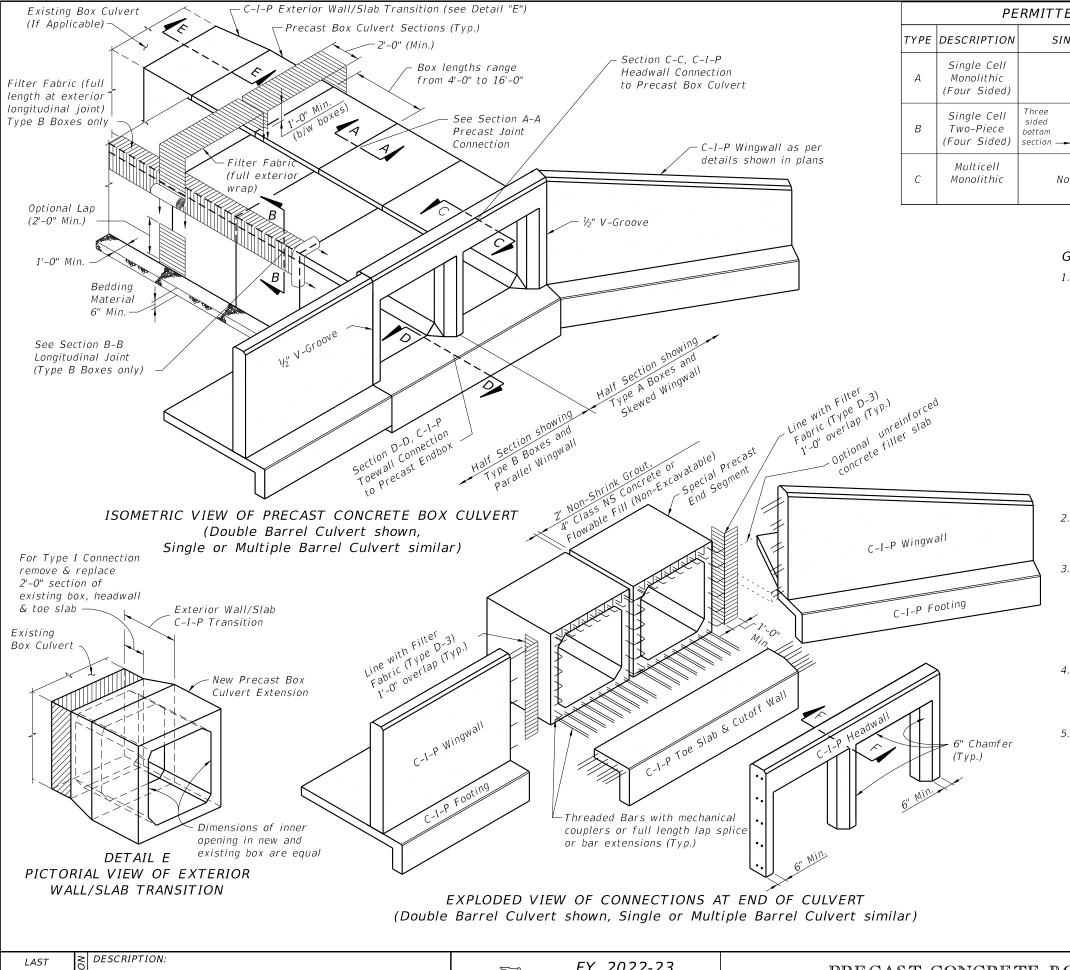






10/1/2021





PERMITTED PRECAST ALTERNATE BOX SECTIONS SINGLE BARREL MULTIPLE BARRELS DESIGN NOTES Index 400-292 Contractor Design section Contractor Design Contractor Design Not Applicable

### **GENERAL NOTES:**

1. Specifications:

General:

FDOT Standard Specifications for Road and Bridge Construction, Section 410 (current edition, and supplements thereto).

Concrete (Precast):

Class III or Class II Modified (5,000 psi) for slightly aggressive environments.

Class IV (5,500 psi) for moderately to extremely aggressive environments.

Concrete (Cast-In-Place):

Class II (3,400 psi) for slightly aggressive environments. Class IV (5,500 psi) for moderately to extremely aggressive environments.

Reinforcing Steel:

Maintain minimum clearance of 2" for slightly and moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. Equal area substitution of welded wire (WWR) reinforcement is permitted.

- 2. Work this Index with the Cast-In-Place Concrete Box Culvert Details and Data Tables shown in the plans, Index 400-289 and the Precast Concrete Box Culverts shown in the shop drawings.
- 3. All joints between precast sections must be tongue & groove with joint sealant. Joints between cast-in-place & precast sections shall have longitudinal reinforcing extending from top, bottom & both side slabs of the precast box tied to the cast-in-place reinforcement. Single barrel culverts may have precast headwalls cast integrally with the end segment when approved by the Engineer.
- 4. Extension of existing multiple barrel box culverts with multiple single cell precast box culverts is not permitted unless approved by the District Structures Engineer. Full transition details must be shown in the shop drawings when approved.
- 5. Culverts larger than the specified size may be substituted with no additional payment to the Contractor. Substitution must be approved by the Engineer, minimum earth cover and invert elevations shown in the Contract Documents must be maintained.

**REVISION** 01/01/11

**FDOT** 

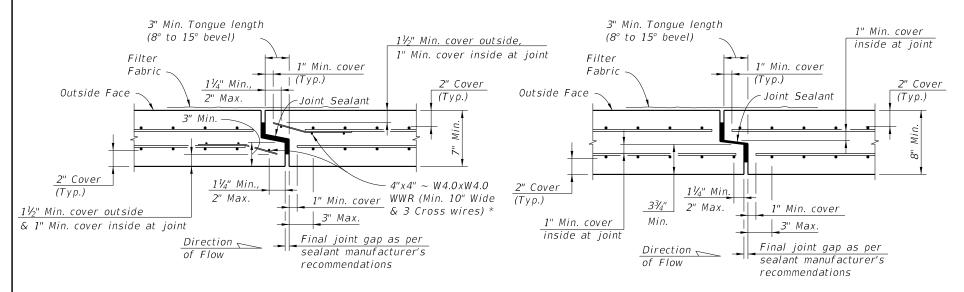
FY 2022-23 STANDARD PLANS

- SUPPLEMENTAL DETAILS

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SHEET 1 of 5

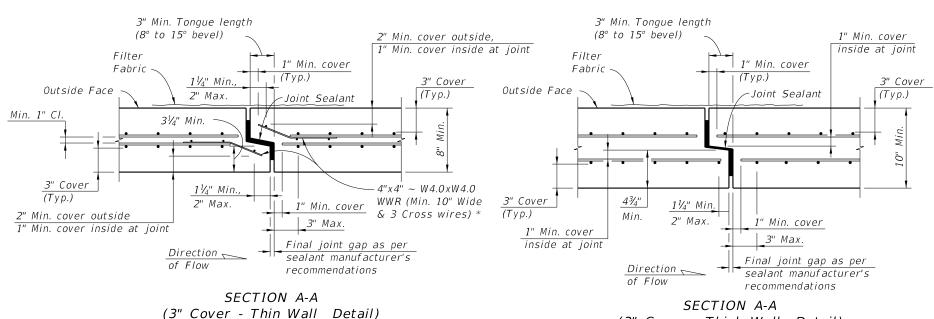
400-291



SECTION A-A (2" Cover - Thin Wall Detail)

SECTION A-A (2" Cover - Thick Wall Detail)

> NOTE: Bottom Slab Joints in Type B Boxes may be single tongue & groove joints as shown in Section A-A when the Top Slab Joints are oriented as shown in Schematic "A".



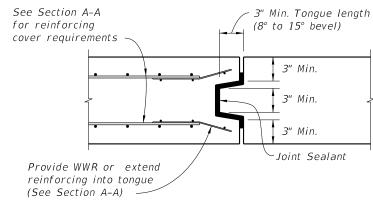
reinforcing utilizes WWR, extend wall and slab reinforcing into the joint and bend to maintain cover in lieu of 4"x4" ~ W4.0xW4.0 WWR at joint. Transverse wire in tongue may be cut at corners

\* At the Contractor's option when the box culvert

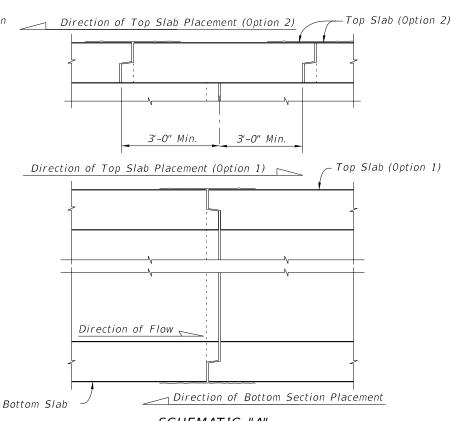
of box to allow bending of the WWR.

(3" Cover - Thick Wall Detail)

PRECAST SEGMENT TO SEGMENT TONGUE & GROOVE TRANSVERSE JOINTS ==



ALTERNATE BOTTOM SLAB TRANSVERSE JOINT TYPICAL SECTION (DOUBLE-SIDED TONGUE & GROOVE JOINT) (All reinforcing not shown for clarity)



SCHEMATIC "A" TYPE B BOX SECTION PLACEMENT FOR SINGLE TONGUE & GROOVE JOINTS

= TWO-PIECE PRECAST SEGMENT ADDITIONAL JOINT DETAILS (TYPE B BOX)

REVISION 07/01/15

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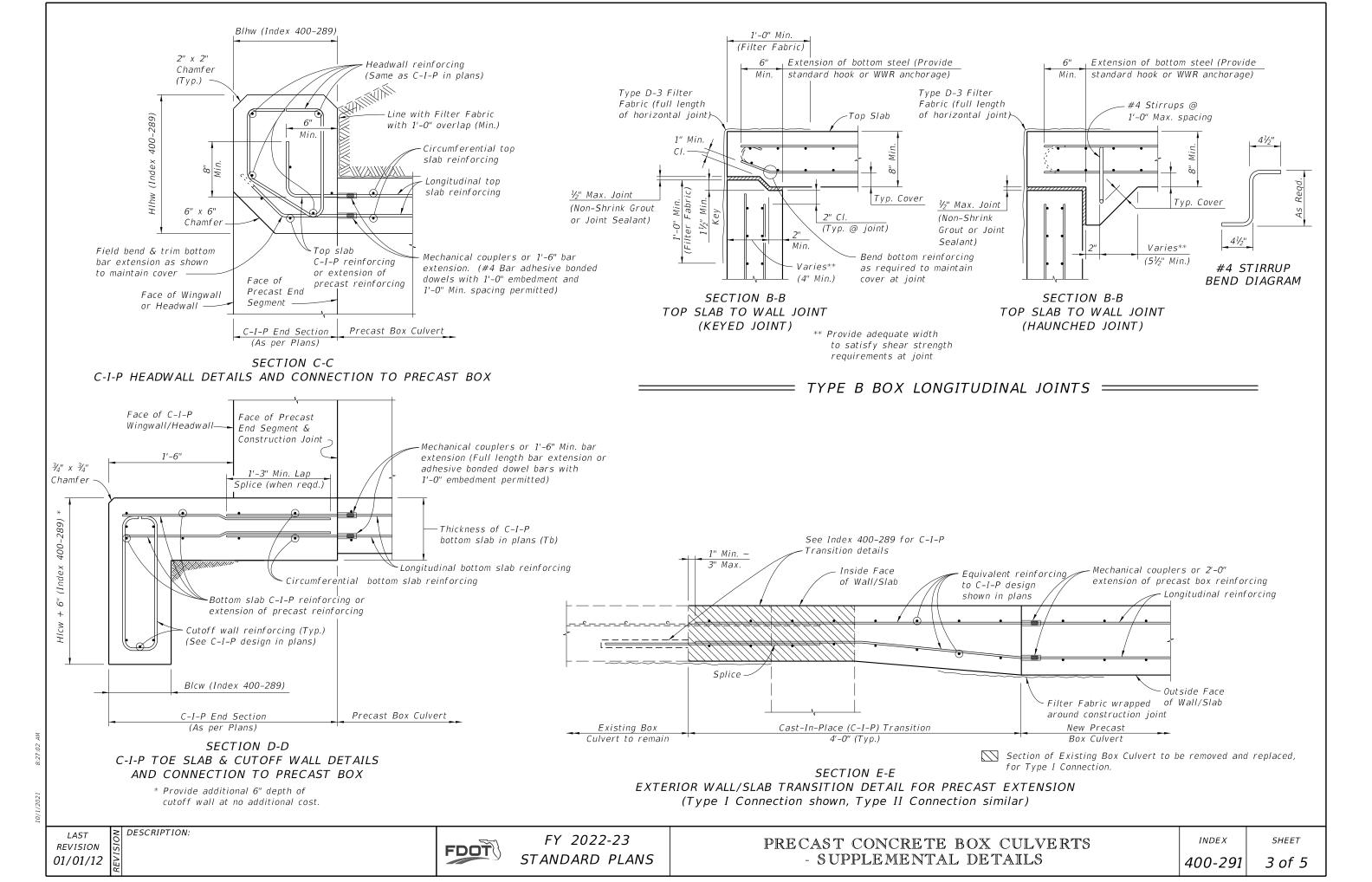
FY 2022-23 STANDARD PLANS PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

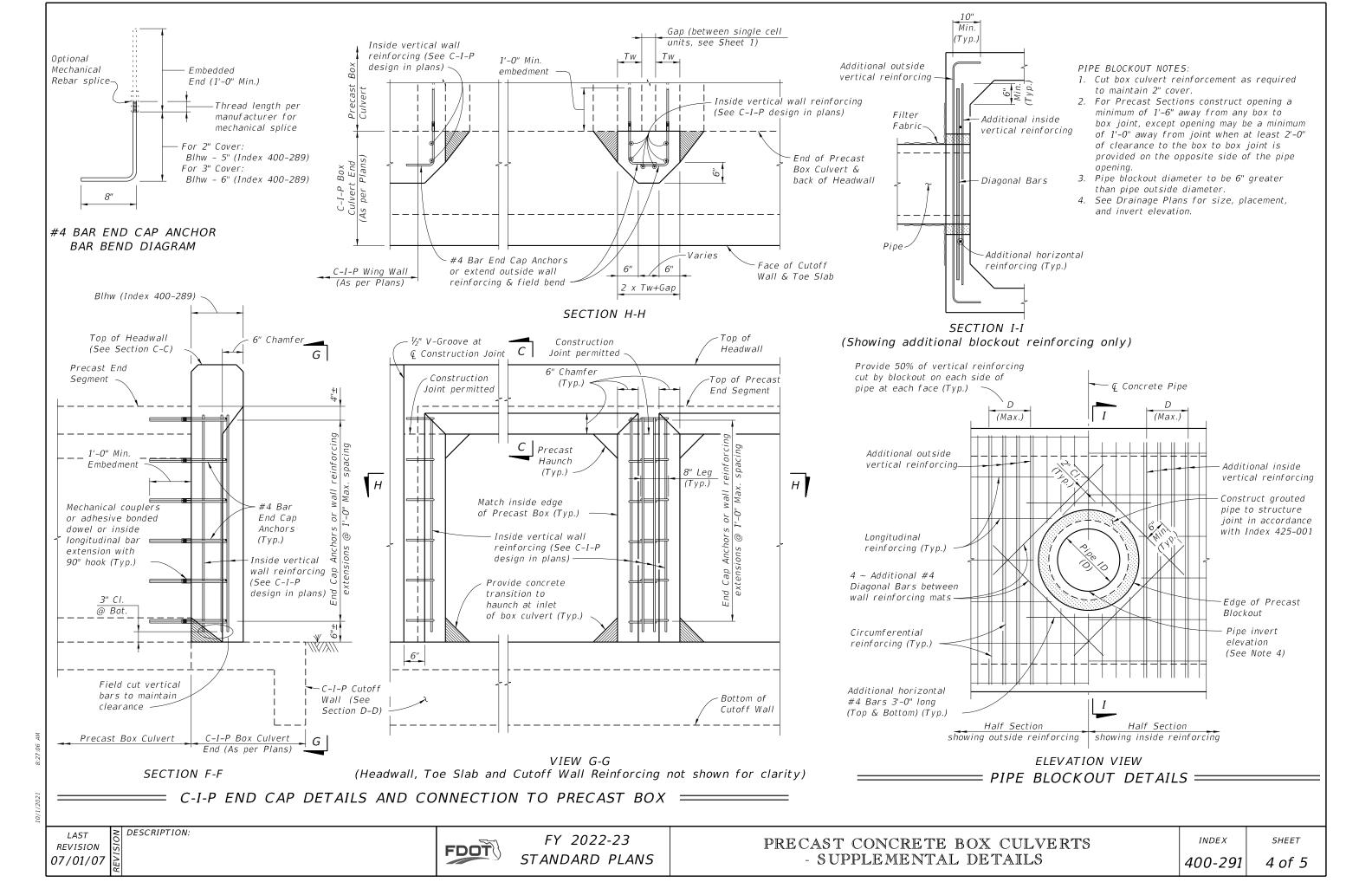
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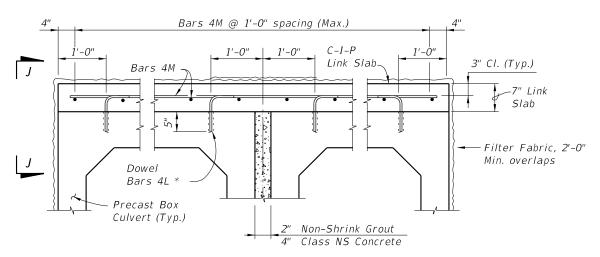
SHEET

400-291 2 of 5

DESCRIPTION:

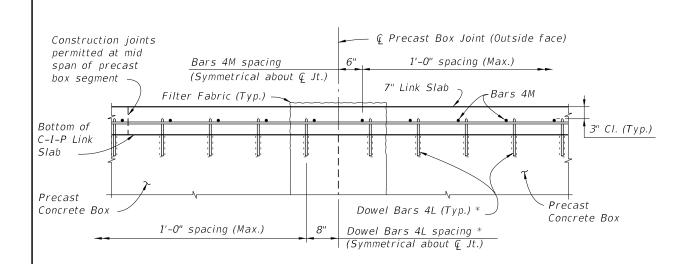






LINK SLAB TYPICAL SECTION (Multiple Barrel Culvert shown, Single Barrel Culvert similar)

\* Install dowels with an Adhesive Bonding Material System in accordance with Specification Section 416. The Contractor may substitute mechanical couplers in lieu of adhesive bonded dowels. Shift dowels to clear box culvert reinforcing.



VIEW J-J

### LINK SLAB NOTES:

1. Provide a Cast-In-Place Link Slab to ensure uniform joint opening of precast box culverts when the differential settlement shown in the plans exceeds the following limits, except that a Link Slab is not required for differential settlements less than 1/2".

$$\Delta Y \leq \frac{(L)^2}{760 \times R \times W}$$

#### Where:

 $\Delta Y = Maximum Long-Term Differential Settlement (ft.)$ 

R = Exterior height of Box Culvert (ft.)

W = Length of Box Culvert Segments (ft.)

L = Effective length for single curvature deflection (ft.)

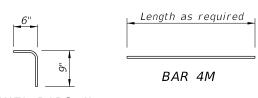
2. Extend Link Slab to back face of headwalls and to limits of existing box culverts for extensions.

ESTIMATED LINK SLA	B QUANTI	TIES
ITEM	UNIT	QUANTITY
Class II or IV Concrete (Culvert)	CY/SF	0.0216
Reinforcing Steel (Roadway)	Lb./SF	1.52

NOTE: Estimated quantities are based the plan area of precast box slabs, and are provided for information only. No additional payment will be made for Link Slabs where these are required for the precast box culverts.

	BILL OF RE	INFORCING STEE	<u>'</u>
MARK	SIZE	NO. REQ'D	LENGTH
L	4	2 per Barrel/Ft.	1'-3"
М	4	As Reqd.	As Reqd.

### REINFORCING STEEL BENDING DIAGRAMS



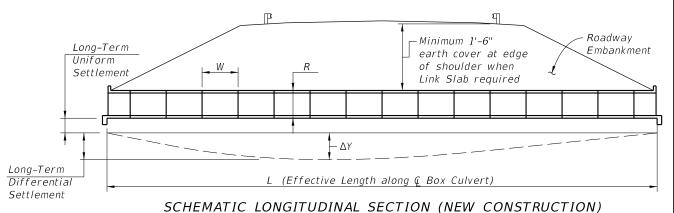
DOWEL BARS 4L

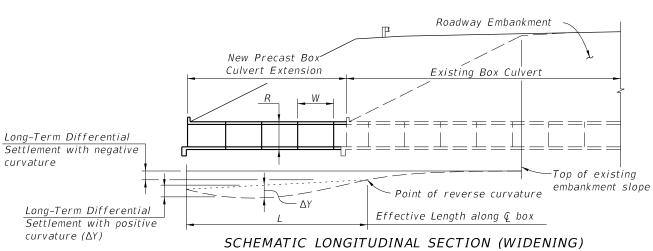
#### NOTES:

- 1. All bar dimensions are out to out.
- 2. Lap splice length for Bars 4M is 1'-4" minimum.

#### DESIGN NOTE:

1. Link Slab required when joint openings from differential settlement exceed 1/8" as determined in Link Slab Note 1.





# = DIFFERENTIAL SETTLEMENT COUNTERMEASURES FOR PRECAST BOX CULVERTS =

REVISION 01/01/09

DESCRIPTION:

**FDOT** 

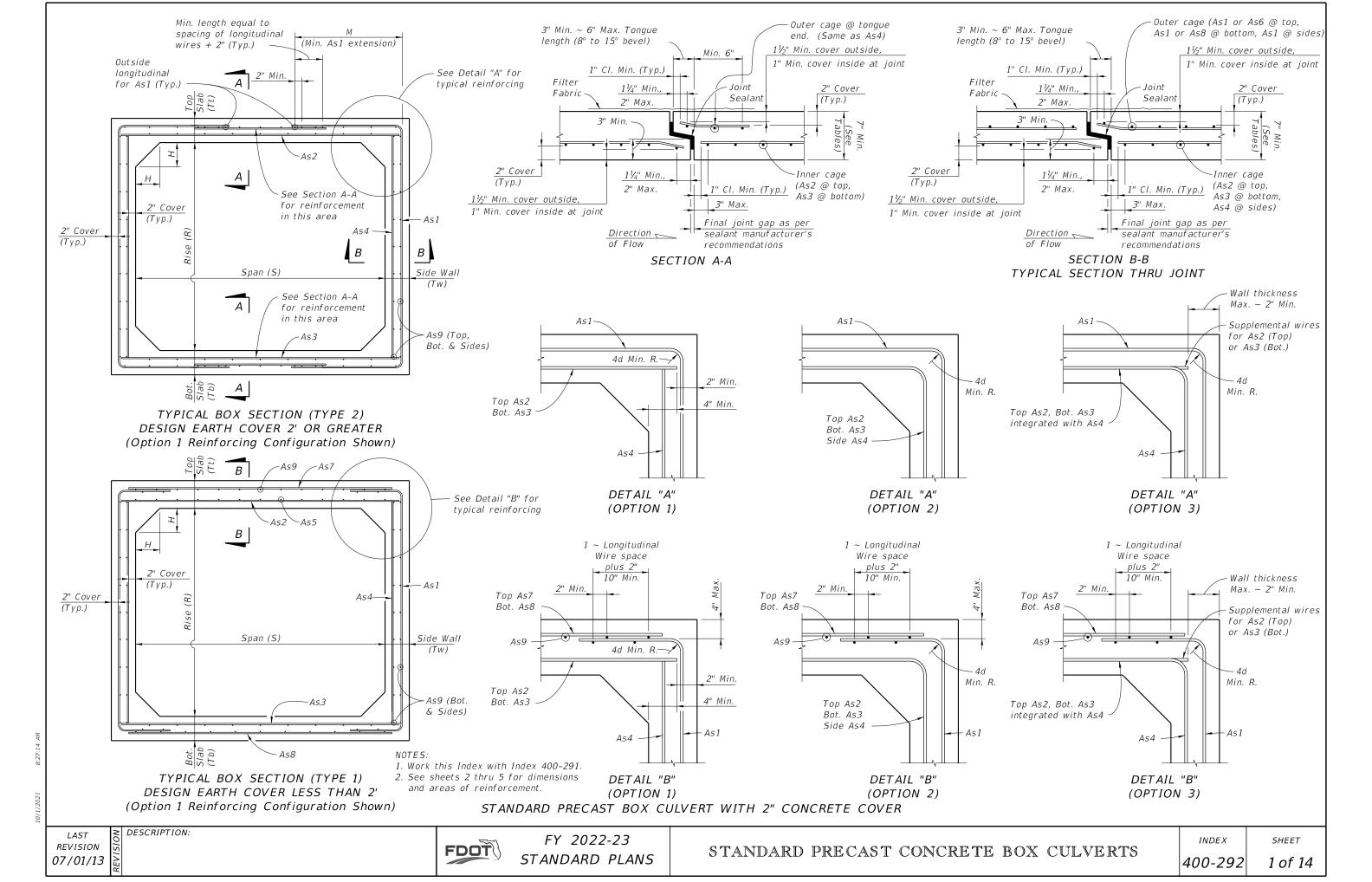
FY 2022-23 STANDARD PLANS

PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

INDEX

SHEET

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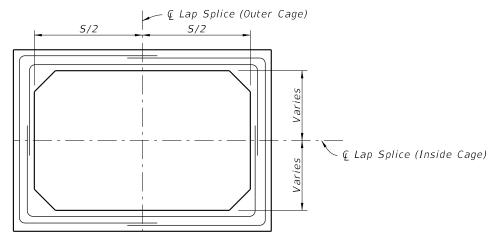


### **GENERAL NOTES:**

- 1. These precast designs may be substituted for cast-in-place box culverts designed to AASHTO LRFD Bridge Design Specifications, 4th Edition. Designs are based on the design criteria shown in FDOT Structures Design Guidelines.
- 2. Loading: HL-93 & any fill heights between the minimum & maximum shown.
- 3. Only one design of precast box culvert is to be used for any installation.
- 4. Reinforcing steel must consist of smooth or deformed welded wire reinforcement (WWR) meeting the requirements of Specification Section 931. Longitudinal reinforcement may consist of reinforcing bars meeting the requirements of Specification Section 931. Minimum cover must be 2" for slightly or moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. The spacing of circumferential wires must not be less than 2" nor more than 4". The spacing of longitudinal wires or bars must not be more than 8".
- 5. As9 longitudinal wires must have a minimum cross-sectional area of 40% of the circumferential wires, but not less than a W2.5 or D4.0 for WWR, or #3 bars for deformed bars.
- 6. Welding of reinforcement must be limited to the locations shown in ASTM C1577 and in accordance with ANSI/AWS D1.4 "Structural Welding Code Reinforcing Steel".
- 7. For alternate reinforcing configuration Options 2 and 3 shown in Detail "A" and "B" (Sheet 1), As1 may be extended to the middle of either slab and lap spliced with As7 and As8. As4 may be lap spliced at any location or connected to As2 or As3 at corners by welding.
- 8. Haunch dimensions may vary between the minimum and maximum dimensions shown in the Design Tables but only one haunch dimension must be used within the full length of the box culvert installation.

TABLE	1A	STANI	DARD	PRECA	ST BOX CU	LVERT	T DES	SIGNS	(2" (	COVE	R) - 3	′&	4' SP.	ANS
SPAN x RISE (S) (R)	TOP	BOT.	SIDE	HAUNCH	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		15		As1 EXT. LENGTH (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.17	0.29	0.21	0.17	0.17	0.17	0.17		-
				4	2' - <3'	0.13	0.28	0.21	0.09	-	-	-		31
					3' - <5'	0.09	0.17	0.17	0.09	-	-	-		31
					5' - 10'	0.09	0.17	0.17	0.09	-	-	-		31
3' x 3'	7	7	7	to	15'	0.09	0.17	0.17	0.09	-	-	-		31
					20'	0.12	0.17	0.17	0.09	-	-	-		31
					25'	0.14	0.18	0.18	0.09	-	-	-		31
				8	30'	0.17	0.21	0.22	0.09	-	-	-		31
					35'	0.19	0.25	0.25	0.09	-	-	-		31
					0.33' - <2'	0.19	0.38	0.26	0.17	0.19	0.17	0.19	e 5	-
				4	2' - <3'	0.19	0.38	0.26	0.09	-	-	-	Note	38
				4	3' - <5'	0.14	0.20	0.22	0.09	-	-	-		38
4' x 3'	7	7	7	to	5' - 10'	0.11	0.17	0.17	0.09	-	-	-	General	38
4 1 2	,	′	′	10	15'	0.15	0.17	0.18	0.09	-	-	-	ene	38
				8	20'	0.20	0.23	0.23	0.09	-	-	-		38
					25'	0.24	0.28	0.29	0.09	-	-	-	See	38
					30'	0.29	0.34	0.35	0.09	-	-	-		38
					0.33' - <2'	0.19	0.41	0.28	0.17	0.21	0.17	0.19		_
				4	2' - <3'	0.19	0.41	0.28	0.09	-	-	-		38
				"	3' - <5'	0.14	0.21	0.24	0.09	-	-	-		38
4' x 4'	7	7	7	to	5' - 10'	0.12	0.17	0.17	0.09	-	-	-		38
4 4 4	/	′	′	10	15'	0.16	0.19	0.20	0.09	-	-	-		38
				8	20'	0.21	0.25	0.25	0.09	-	-	-		38
					25'	0.26	0.31	0.32	0.09	-	-	-		38
					30'	0.31	0.37	0.38	0.09	-	_	-		38

- 9. Submittal of redesign calculations are not required for any increase to the slab and/or wall thickness when the minimum reinforcement areas shown in the Design Tables are provided.
- 10. For Design Earth Cover greater than 10 feet, the Contractor may interpolate the required areas of reinforcement and slab or wall thickness. Interpolated areas of reinforcement, slab or wall thickness must be approved by the Engineer.
- 11. Minimum length of precast box segments is 4 feet and maximum length is 16 feet.
- 12. See Index 400-291 for connections to wingwalls, headwalls and other general details.



SCHEMATIC OF LAP SPLICE LOCATIONS FOR OPTION 2 & 3 REINFORCING CONFIGURATIONS

TABL	.E 1B	- STA	ANDAI	RD PRE	CAST BOX	CULVE	ERT D	ESIG	NS (2	" COV	/ER) -	- 3'	& 4'.	SPANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.20	0.26	0.32	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.16	0.25	0.31	0.10	-	-	-		31
					3' - <5'	0.10	0.20	0.20	0.10	-	-	-		31
					5' - 10'	0.10	0.20	0.20	0.10	-	-	-		31
3' x 3'	8	8	8	to	15'	0.10	0.20	0.20	0.10	-	-	-		31
					20'	0.10	0.20	0.20	0.10	-	-	-		31
					25'	0.11	0.20	0.20	0.10	-	-	-		31
				8	30'	0.13	0.20	0.20	0.10	-	-	-		31
					35'	0.15	0.21	0.21	0.10	-	-	-	1	31
					0.33' - <2'	0.20	0.31	0.22	0.20	0.20	0.20	0.20	5 5	-
				4	2' - <3'	0.12	0.31	0.22	0.10	-	-	-	Note	38
				4	3' - <5'	0.12	0.20	0.20	0.10	-	-	-	\ <u>\</u>	38
4' x 3'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	General	38
4 ^ 3	0	0	0	"	15'	0.12	0.20	0.20	0.10	-	-	-	епе	38
				8	20'	0.16	0.20	0.20	0.10	-	-	-		38
					25'	0.19	0.24	0.24	0.10	-	-	-	See	38
					30'	0.22	0.28	0.29	0.10	-	-	-	] ,	38
					0.33' - <2'	0.20	0.33	0.24	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.17	0.33	0.24	0.10	-	-	-		38
				4	3' - <5'	0.12	0.20	0.20	0.10	-	-	-		38
4' x 4'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-		38
4 ^ 4	0	U		"	15'	0.13	0.20	0.20	0.10	-	-	-		38
				8	20'	0.16	0.21	0.22	0.10	-	-	-		38
					25'	0.20	0.26	0.27	0.10	-	-	-		38
					30'	0.23	0.31	0.32	0.10	-	-	-		38

NOTES: 1. See Sheet 1 for Reinforcing Details and dimension locations.

2. See Sheet 14 for WWR Bending Diagram.

07/01/15

TABL	LE 2A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT D	DESIG	NS (2	?" COI	/ER)	- 5'	& 6'	SPANS
PAN x RISE		/ WAL	L THIC	KNESS	DESIGN			R		RCEMEN		ıS		As1 EX
S) (R)	TOP	BOT.	l	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGT
(F+ )	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.31	0.48	0.42	0.17	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.48	0.42	0.09	-	_	_		45
				,	3' - <5'	0.20	0.27	0.27	0.09	_	_	_		36
5' x 3'	7	7	7	+0	5' - 10'	0.17	0.19	0.21	0.09	_	_	_		36
<i>3</i>	′	′	′	to	15'	0.24	0.25	0.25	0.09	_	_	_		35
					20'	0.32	0.23	0.23	0.09	_				35
				8	25'					_		_		
						0.39	0.41	0.42	0.09	-	-	-		35
					30'	0.47	0.50	0.50	0.09	-	-	-		35
					0.33' - <2'	0.30	0.51	0.45	0.17	0.23	0.21	0.30		
				4	2' - <3'	0.30	0.51	0.45	0.09	-	ı	1		45
					3' - <5'	0.18	0.30	0.29	0.09	-	-	-		45
5' x 4'	7	7	7	to	5' - 10'	0.17	0.21	0.23	0.09	_	-	-		36
					15'	0.24	0.27	0.28	0.09	-	-	_		35
				8	20'	0.31	0.36	0.37	0.09	_	_	_		35
					25'	0.39	0.45	0.46	0.09	_	_	_		35
					30'	0.46	0.55	0.56	0.09	_	_	_		35
					0.33' - <2'	0.30	0.53	0.48	0.17	0.24	0.21	0.30		-
				4	2' - <3'	0.29	0.53	0.48	0.09	-	-	-		45
					3' - <5'	0.19	0.31	0.31	0.09	-	-	_		45
5' x 5'	7	7	7	to	5' - 10'	0.19	0.22	0.25	0.09	-	-	-		45
					15'	0.26	0.29	0.31	0.09	-	-	-		36
				8	20'	0.34	0.39	0.40	0.09	-	_	_		35
					25'	0.41	0.49	0.50	0.09	-	_	_		35
					30'	0.49	0.59	0.61	0.09	_	_	_		35
	7.5	7	7				0.54			0.22	0.25	0.20	10	
	7.5	/	/		0.33' - <2'	0.39		0.48	0.17			0.39	7	-
				4	2' - <3'	0.39	0.58	0.49	0.09	-	-	-	Note	43
					3' - <5'	0.28	0.36	0.36	0.09	-	-	-	>	39
6' x 3'	7	7	7	to	5' - 10'	0.25	0.26	0.28	0.09	-	-	-	General	39
					15'	0.36	0.34	0.34	0.09	_	_	_	lei	38
				12	20'	0.47	0.46	0.46	0.09	-	-	_	3e	38
	7	7.5	7		25'	0.59	0.57	0.55	0.09	_	_	_		38
	8	8	7		30'	0.60	0.64	0.64	0.09	_	_	_	See	38
	7.5	7	7		0.33' - <2'	0.37	0.58	0.52	0.17	0.24	0.23	0.37		- 30
	7.5													12
				4	2' - <3'	0.37	0.61	0.53	0.09	-	-	1-		43
					3' - <5'	0.26	0.39	0.39	0.09	-	-	-		39
6' x 4'	7	7	7	to	5' - 10'	0.24	0.28	0.31	0.09	-	-	-		39
					15'	0.35	0.37	0.38	0.09	-	-	-		38
		L		12	20'	0.46	0.50	0.50	0.09	-	-	_		38
	7	7.5	7		25'	0.56	0.63	0.60	0.09	-	-	_		38
	8	8	7	1	30'	0.58	0.69	0.69	0.09	-	_	_		38
	7.5	7	7		0.33' - <2'	0.36	0.60	0.56	0.17	0.25	0.22	0.36		-
	1.5	<del>- ' -</del>	<del>  '</del>		2' - <3'	0.36	0.64	0.56	0.17	-	-	-		43
				4										
CI =:	_	_	_		3' - <5'	0.26	0.410	0.42	0.09	-	-	_		43
6' x 5'	7	7	7	to	5' - 10'	0.25	0.30	0.33	0.09	-	-	_		39
					15'	0.34	0.40	0.41	0.09	-	-	-		38
				12	20'	0.46	0.54	0.54	0.09	-	-	-		38
	7	7.5	7		25'	0.56	0.67	0.65	0.09	-	-	-		38
	8	8	8	]	30'	0.60	0.74	0.74	0.09	_	-	_		38
	7.5	7	7		0.33' - <2'	0.36	0.63	0.59	0.17	0.26	0.22	.036		_
	1.5				2' - <3'		0.67	0.59	0.17	-	-			52
				4		0.35								
-	_	_	_		3' - <5'	0.27	0.43	0.44	0.09	-	-	_		52
6' x 6'	7	7	7	to	5' - 10'	0.27	0.32	0.35	0.09	-	-	-		43
					15'	0.38	0.43	0.44	0.09	-	-	-		39
				12	20'	0.50	0.57	0.59	0.09	-	-	-		39
	7	7.5	7	1	25'	0.60	0.72	0.70	0.09	-	-	-		38
	8	8	7	1	30'	0.67	0.78	0.79	0.09	_	_	_		38
				1		0.07	1 0., 0	L J., J	1 0.00				1	1 50

		- 317	ANDA	KD PKE	CAST BOX	CULVI	ERI L	DESIG	NS (2	" CO	VER)	- 5'	& 6'	<i>SPANS</i>
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EX7
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(F+ )	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
ļ					0.33' - <2'	0.26	0.39	0.36	0.20	0.20	0.20	0.26		_
				4	2' - <3'	0.26	0.39	0.36	0.10	-	-	-		45
					3' - <5'	0.16	0.23	0.24	0.10	-	-	-		36
5' x 3'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	-	-	-		36
					15'	0.19	0.21	0.22	0.10	-	-	-		35
				8	20'	0.24	0.28	0.28	0.10	-	-	-		35
					25'	0.30	0.34	0.35	0.10	-	-	-		35
					30'	0.36	0.41	0.41	0.10	_	_	_		35
					0.33' - <2'	0.25	0.42	0.38	0.20	0.20	0.20	0.25		_
				4	2' - <3'	0.25	0.42	0.38	0.10	_	_	_		45
				4	3' - <5'	0.16	0.25	0.25	0.10	_	_	_		45
5' x 4'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	_	_	_		36
<i>3</i>		0		10	15'	0.19	0.23	0.24	0.10	_	_	_		35
					20'	0.13	0.30	0.24	0.10	_	_	_		35
				8	25'	0.24	0.37	0.31	0.10					35
					30'	0.35	0.37	0.38	0.10	-	=	-		35
										- 0.20	- 0.20	- 0.25		
					0.33' - <2'	0.25	0.44	0.41	0.20	0.20	0.20	0.25		-
				4	2' - <3'	0.25	0.44	0.41	0.10	-	-	-		45
E, E.					3' - <5'	0.16	0.26	0.27	0.10	-	-	-		45
5' x 5'	8	8	8	to	5' - 10'	0.15	0.20	0.22	0.10	-	-	-		45
					15'	0.20	0.25	0.26	0.10	-	-	-		36
				8	20'	0.26	0.32	0.33	0.10	-	-	-		35
					25'	0.32	0.40	0.41	0.10	-	-	-		35
					30'	0.37	0.48	0.49	0.10	-	-	-		35
					0.33' - <2'	0.32	0.47	0.41	0.20	0.20	0.25	0.32	5	_
				4	2' - <3'	0.32	0.47	0.41	0.10	-	-	-	Note	43
					3' - <5'	0.23	0.30	0.31	0.10	-	-	-	N	39
6' x 3'	8	8	8	to	5' - 10'	0.19	0.22	0.24	0.10	-	-	-	General	39
					15'	0.28	0.29	0.29	0.10	-	-	-	ne.	38
				12	20'	0.36	0.38	0.38	0.10	-	-	-	Ge.	38
					25'	0.45	0.47	0.47	0.10	-	-	-	See	38
					30'	0.54	0.57	0.57	0.10	-	-	-	Sé	38
					0.33' - <2'	0.31	0.50	0.44	0.20	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.50	0.44	0.10	_	_	_		43
				,	3' - <5'	0.23	0.32	0.34	0.10	_	_	_		39
6' x 4'	8	8	8	to	5' - 10'	0.19	0.24	0.26	0.10	-	-	-		39
					15'	0.27	0.31	0.32	0.10	_	_	_		38
				12	20'	0.35	0.41	0.32	0.10	_	_	_		38
				12	25'	0.33	0.41	0.41	0.10	_		_		38
					30'	0.43	0.62	0.62	0.10	_	_	_		38
					0.33' - <2'	0.32	0.52							-
								0.47	0.20	0.22	0.22	0.30		
				4	2' - <3'	0.30	0.52	0.47	0.10	-	-	-		43
6' F'	0	0	,		3' - <5'	0.22	0.34	0.36	0.10	-	_	-		
6' x 5'	8	8	8	to	5' - 10'	0.20	0.26	0.28	0.10	-	-	-		39
					15'	0.27	0.33	0.34	0.10	-	-	-		38
				12	20'	0.36	0.44	0.45	0.10	-	-	-		38
					25'	0.44	0.55	0.55	0.10	-	-	-		38
					30'	0.52	0.66	0.67	0.10	-	-	-		38
					0.33' - <2'	0.30	0.54	0.50	0.20	0.22	0.22	0.30		-
				4	2' - <3'	0.30	0.54	0.50	0.10	-	-	-		52
					3' - <5'	0.23	0.36	0.38	0.10	-	-	-		52
6' x 6'	8	8	8	to	5' - 10'	0.21	0.27	0.30	0.10	-	-	-		43
					15'	0.29	0.35	0.37	0.10	-	-	-		39
				12	20'	0.38	0.47	0.48	0.10	-	-	-		39
					25'	0.47	0.59	0.60	0.10	-	-	-		38
					30'	0.55	0.70	0.71	0.10	-	_	-		38

≥ DESCRIPTION: LAST REVISION 07/01/13

Т	ABLE	3 - 9	STANI	DARD P	RECAST BO	X CU	LV ER7	DES	IGNS	(2" (	COVER	R) - 7	SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				1	0.33' - <2'	0.37	0.58	0.49	0.20	0.22	0.29	0.37		-
				4	2' - <3'	0.37	0.58	0.49	0.10	-	-	-		43
				4	3' - <5'	0.30	0.40	0.42	0.10	-	-	_		43
7' x 4'	8	8	8	to	5' - 10'	0.26	0.30	0.33	0.10	-	-	-		43
				1.2	15'	0.37	0.40	0.40	0.10	-	-	-		41
				12	20'	0.49	0.53	0.53	0.10	-	-	-		41
	8	8	8	7 to	25'	0.60	0.67	0.66	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.68	0.79	0.78	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.60	0.53	0.20	0.23	0.28	0.36		_
				4	2' - <3'	0.36	0.60	0.53	0.10	-	-	-		47
				4	3' - <5'	0.30	0.42	0.45	0.10	-	-	-		43
7' x 5'	8	8	8	to	5' - 10'	0.26	0.32	0.35	0.10	-	-	-		43
				1.2	15'	0.37	0.43	0.44	0.10	-	-	-	5	41
				12	20'	0.48	0.57	0.57	0.10	-	-	-	te	41
	8	8	8	7 to	25'	0.60	0.72	0.72	0.10	-	-	-	No	41
	8.5	8.5	8	12	30'	0.67	0.84	0.84	0.10	-	-	-	le.	41
				4	0.33' - <2'	0.36	0.63	0.56	0.20	0.24	0.27	0.36	General Note	-
				4	2' - <3'	0.36	0.63	0.56	0.10	-	-	-	(ee)	59
				4.0	3' - <5'	0.29	0.44	0.47	0.10	-	-	-	See	47
7' x 6'	8	8	8	to	5' - 10'	0.27	0.34	0.37	0.10	-	-	-	56	43
				12	15'	0.38	0.46	0.46	0.10	-	-	-		41
				12	20'	0.49	0.60	0.61	0.10	-	-	-		41
	8	8	8	7 to	25'	0.61	0.76	0.76	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.69	0.89	0.89	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.65	0.58	0.20	0.25	0.27	0.36		_
				4	2' - <3'	0.36	0.65	0.58	0.10	-	-	-		59
					3' - <5'	0.30	0.46	0.50	0.10	-	-	-		59
7' x 7'	8	8	8	to	5' - 10'	0.30	0.35	0.50	0.10	-	-	-		47
				12	15'	0.41	0.48	0.50	0.10	-	-	-		43
				12	20'	0.53	0.64	0.65	0.10	-	-	-		43
	8	8	8	7 to	25'	0.65	0.80	0.81	0.10	-	-	-		43
	8.5	9	8	12	30'	0.72	0.92	0.91	0.10	-	-	-		41

T	ABLE	4 - 9	STANL	DARD P	RECAST BO	X CU	LVERT	DES	IGNS	(2" C	COVE	?) - 8	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
	9	8.5	8	4	0.33' - <2'	0.40	0.60	0.52	0.20	0.22	0.28	0.39		_
				4	2' - <3'	0.45	0.66	0.54	0.10	-	-	-		50
				to	3' - <5'	0.39	0.48	0.50	0.10	-	-	-		50
8' x 4'	8	8	8	1 10	5' - 10'	0.34	0.38	0.40	0.10	-	-	-		45
				12	15'	0.49	0.51	0.50	0.10	-	-	-		41
				12	20'	0.65	0.68	0.66	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.76	0.83	0.80	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.79	0.94	0.92	0.10	-	-	-		41
	9	8.5	8	4	0.33' - <2'	0.38	0.65	0.59	0.20	0.22	0.30	0.37		-
				4	2' - <3'	0.43	0.69	0.58	0.10	-	-	-		50
				to	3' - <5'	0.37	0.51	0.53	0.10	-	-	-		45
8' x 5'	8	8	8	10	5' - 10'	0.33	0.41	0.42	0.10	-	-	-		45
				12	15'	0.48	0.54	0.53	0.10	-	-	-		41
				12	20'	0.63	0.73	0.70	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.74	0.88	0.86	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.77	1.00	0.98	0.10	-	-	-		41
	9	9	8	4	0.33' - <2'	0.32	0.65	0.58	0.20	0.23	0.25	0.31	5	-
				] 4	2' - <3'	0.42	0.71	0.61	0.10	-	-	-	Note	50
				t o	3' - <5'	0.37	0.54	0.56	0.10	-	-	-		50
8' x 6'	8	8	8	to	5' - 10'	0.34	0.43	0.45	0.10	-	-	-	General	45
				12	15'	0.49	0.57	0.57	0.10	-	-	-	ner	41
				12	20'	0.64	0.77	0.76	0.10	-	-	-	<i>Ge</i>	41
	8.5	8.5	8	8 to	25'	0.74	0.94	0.92	0.10	-	-	-	See	41
	9.5	9.5	8	12	30'	0.78	1.05	1.04	0.10	-	-	-	Š	41
	9	9	8	4	0.33' - <2'	0.31	0.67	0.60	0.20	0.24	0.24	0.31		-
				4	2' - <3'	0.42	0.74	0.64	0.10	-	-	-		55
				+0	3' - <5'	0.37	0.56	0.59	0.10	-	-	-		55
8' x 7'	8	8	8	to	5' - 10'	0.36	0.45	0.47	0.10	-	-	-		50
				12	15'	0.51	0.61	0.61	0.10	-	-	-		45
				12	20'	0.66	0.81	0.80	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.78	0.98	0.97	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.84	1.10	1.09	0.10	-	_	-		41
	9	9	8	4	0.33' - <2'	0.32	0.68	0.62	0.20	0.24	0.25	0.32		-
				4	2' - <3'	0.43	0.76	0.67	0.14	-	-	-		65
				t o	3' - <5'	0.38	0.58	0.61	0.14	-	-	-		65
8' x 8'	8	8	8	to	5' - 10'	0.39	0.46	0.50	0.13	-	-	-		55
				12	15'	0.55	0.64	0.65	0.10	-	-	-		45
				12	20'	0.71	0.86	0.85	0.10	-	-	-		45
	8.5	8.5	8	8 to	25'	0.84	1.03	1.02	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.93	1.15	1.15	0.10	-	-	-		41

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- See Sheet 2 for General Notes.
   See Sheet 14 for Welded Wire Reinforcement Bending Diagram.



SHEET

SPAN x RISE	SLAE	3 / WAL	L THIC	KNESS	DESIGN			R	EINFOF	RCEMEN	T AREA	15		As1 EXT
(S) (R)	TOP	BOT.	_	HAUNCH					(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M) (in.)
(1 1.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	As9	(111.)
	9.5	9.5	9	4	0.33' - <2'	0.41	0.62	0.53	0.22	0.23	0.34	0.38		-
					2' - <3'	0.44	0.65	0.54	0.11	-	-	-		54
	_			to	3' - <5'	0.39	0.53	0.51	0.11	-	-	-		49
9' x 5'	9	9	9		5' - 10'	0.35	0.42	0.44	0.11	-	-	-		49
				12	15'	0.50	0.56	0.55	0.11	-	-	-		44
					20'	0.65	0.75	0.73	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.77	0.92	0.90	0.11	-	-	-		44
	10.5	11	9	12	30'	0.81	1.05	1.02	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.38	0.64	0.56	0.23	0.23	0.33	0.37		_
				,	2' - <3'	0.43	0.67	0.57	0.11	-	-	-		54
				to	3' - <5'	0.37	0.55	0.54	0.11	-	-	_		49
9' x 6'	9	9	9		5' - 10'	0.35	0.45	0.47	0.11	-	-	-		49
				12	15'	0.49	0.60	0.59	0.11	-	-	-		44
				12	20'	0.65	0.80	0.78	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.76	0.98	0.95	0.11	_	_	_		44
	10.5	11	9	12	30'	0.80	1.10	1.08	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.37	0.67	0.59	0.22	0.23	0.32	0.37	5	-
				4	2' - <3'	0.42	0.69	0.60	0.11	-	-	-	Note	59
					3' - <5'	0.37	0.58	0.56	0.11	-	-	_	No	54
9' x 7'	9	9	9	to	5' - 10'	0.36	0.47	0.49	0.11	-	-	_	le	49
				1.0	15'	0.50	0.63	0.63	0.11	-	-	-	ıer	44
				12	20'	0.66	0.84	0.80	0.11	-	-	-	General	44
	9.5	9.5	9	8 to	25'	0.77	1.02	1.00	0.11	-	-	-	See	44
	10.5	11	9	12	30'	0.81	1.15	1.13	0.11	-	-	-	Se	44
	9.5	9.5	9		0.33' - <2'	0.37	0.68	0.61	0.22	0.23	0.31	0.37		_
				4	2' - <3'	0.42	0.71	0.62	0.11	-	-	-		59
					3' - <5'	0.37	0.60	0.59	0.11	_	_	_		59
9' x 8'	9	9	9	to	5' - 10'	0.38	0.49	0.51	0.11	-	_	_		54
					15'	0.53	0.66	0.66	0.11	_	_	_		44
				12	20'	0.68	0.88	0.87	0.11	_	_	_		44
	9.5	9.5	9	8 to	25'	0.81	1.07	1.05	0.11	_	_	_		44
	10.5	11	9	12	30'	0.86	1.20	1.18	0.11	_	_	_		44
	9.5	9.5	9		0.33' - <2'	0.38	0.70	0.63	0.22	0.23	0.32	0.38		
	7.5	7.5	<u> </u>	4	2' - <3'	0.43	0.73	0.65	0.15	-	-	-		72
					3' - <5'	0.43	0.62	0.61	0.15	_	_	_		72
9' x 9'	9	9	9	to	5' - 10'	0.41	0.50	0.53	0.14	_	_	_		59
J , J		_			15'	0.41	0.69	0.70	0.14	_	_	_		49
				12	20'	0.73	0.03	0.91	0.12	_	_	_		49
	9.5	10	9	8 to	25'	0.73	1.11	1.09	0.11	_	_	_		44
	10.5	11	9	12	30'	0.03	1.11	1.09	0.11	<del>-</del>		_		44
	10.5	1 1	<u> </u>	12	1 20	0.93	1.23	1.23	0.11	_				1 44

TABL	E 6 -	STAI	VDARI	O PREC	AST BOX C	ULVE	RT DE	SIGN	5 (2"	COVI	ER) -	10' SI	PANS	
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.46	0.62	0.52	0.24	0.24	0.41	0.45		-
				4	2' - <3'	0.46	0.62	0.52	0.12	_	_	-		58
					3' - <5'	0.42	0.54	0.50	0.12	_	-	-		53
10' x 5'	10	10	10	to	5' - 10'	0.38	0.46	0.49	0.12	-	-	-		52
					15'	0.52	0.59	0.58	0.12	_	_	_		47
				12	20'	0.69	0.78	0.76	0.12	_	_	_		47
	10.5	10.5	10	8 to	25'	0.81	0.97	0.93	0.12	-	-	-		47
	11.5	12	10	12	30'	0.87	1.11	1.11	0.12	_	_	_		47
	11.0				0.33' - <2'	0.44	0.64	0.54	0.24	0.24	0.39	0.44		
				4	2' - <3'	0.44	0.64	0.54	0.12	-	-	0.44		58
					3' - <5'	0.39	0.57	0.52	0.12	_	_	_		52
10' x 6'	10	10	10	to	5' - 10'	0.37	0.37	0.52	0.12	_	_	_		52
10 x 0	10	10	10		15'	0.51	0.48	0.52	0.12	_	_	_		47
				12						-	-	-		47
	10.5	10.5	1.0	0.1	20'	0.67	0.83	0.80	0.12	-	-	-		
	10.5	10.5	10	8 to	25'	0.79	1.02	0.99	0.12	-	-	-		47
	11.5	12	10	12	30'	0.85	1.17	1.14	0.12	-	-	-		47
				4	0.33' - <2'	0.43	0.66	0.57	0.24	0.24	0.38	0.43		-
					2' - <3'	0.43	0.66	0.57	0.12	-	-	-		58
				to	3' - <5'	0.38	0.59	0.55	0.12	-	-	-		58
10' x 7'	10	10	10		5' - 10'	0.37	0.50	0.54	0.12	-	-	-		52
				12	15'	0.52	0.66	0.65	0.12	-	-	-	5	47
				12	20'	0.67	0.87	0.85	0.12	-	-	-	Note	47
	10.5	10.5	10	8 to	25'	0.79	1.07	1.04	0.12	-	-	-	×	47
	11.5	12	10	12	30'	0.84	1.22	1.19	0.12	-	-	-	General	47
				4	0.33' - <2'	0.43	0.68	0.60	0.24	0.24	0.38	0.43	Jer	-
				4	2' - <3'	0.43	0.68	0.60	0.12	_	_	-	Ge.	64
				4 -	3' - <5'	0.38	0.62	0.57	0.12	_	-	-	See	58
10' x 8'	10	10	10	to	5' - 10'	0.38	0.52	0.57	0.12	-	-	-	Se	52
					15'	0.53	0.69	0.68	0.12	_	_	-		47
				12	20'	0.68	0.91	0.89	0.12	_	_	-		47
	10.5	10.5	10	8 to	25'	0.81	1.12	1.09	0.12	_	-	-		47
	11.5	12	10	12	30'	0.86	1.27	1.25	0.12	_	_	_		47
	1				0.33' - <2'	0.43	0.70	0.62	0.24	0.24	0.38	0.43		_
				4	2' - <3'	0.43	0.70	0.62	0.12	-	-			70
					3' - <5'	0.43	0.70	0.62	0.12	_	_	_		64
10' > 0'	10	10	10	to	5' - 10'		0.54	0.59	0.12	_	<del>-</del>	_		58
10' x 9'	10	10	10							_		_		
				12	15'	0.56	0.72	0.72	0.12	-	_	-		52
	10.5	1.1	10	0.1.	20'	0.71	0.95	0.94	0.12	-	-	-		47
	10.5	11	10	8 to	25'	0.82	1.15	1.13	0.12	-	_	-		47
	11.5	12	10	12	30'	0.90	1.32	1.30	0.12	-	-	-		47
				4	0.33' - <2'	0.44	0.71	0.64	0.24	0.24	0.38	0.44		-
					2' - <3'	0.44	0.71	0.64	0.17	-	-	-		79
				to	3' - <5'	0.40	0.65	0.62	0.16	-	=	-		70
10' × 10'	10	10	10		5' - 10'	0.44	0.56	0.61	0.15	-	-	-		64
				12	15'	0.60	0.75	0.76	0.12	-	-	-		52
			<u> </u>	12	20'	0.76	0.99	0.99	0.12	-	-	-		52
	10.5	11	10	8 to	25'	0.86	1.20	1.18	0.12	-	-	-		47
	11.5	12	10	12	30'	0.97	1.36	1.35	0.13	-	-	-		47

- See Sheet 1 for Reinforcing Details and dimension locations.
   See Sheet 2 for General Notes.
   See Sheet 14 for WWR Bending Diagram.

LAST REVISION 07/01/13

≥ DESCRIPTION:

TAB					CAST BOX	CULVE	RT D	ESIG	NS (2	" COV	/ER) -	- 11' S	PANS	
SPAN x RISE		/ WAL			DESIGN			R			T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(F)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.51	0.57	0.47	0.27	0.27	0.45	0.48		-
				, '	2' - <3'	0.51	0.57	0.47	0.14	-	-	-		62
				to	3' - <5'	0.48	0.57	0.46	0.14	-	-	-		62
11' x 4'	11	11	11	10	5' - 10'	0.47	0.50	0.50	0.14	-	-	-		55
				12	15'	0.59	0.58	0.56	0.14	-	-	-		55
				12	20'	0.77	0.77	0.74	0.14	-	_	-		55
	11.5	11.5	11	8 to	25'	0.92	0.95	0.91	0.14	-	-	-		55
	13	13	11	12	30'	0.94	1.09	1.06	0.14	-	-	-		55
				4	0.33' - <2'	0.45	0.62	0.52	0.27	0.27	0.41	0.45		_
				7	2' - <3'	0.45	0.62	0.52	0.14	-	-	-		62
				to	3' - <5'	0.42	0.58	0.51	0.14	-	_	_		55
11' x 6'	11	11	11	10	5' - 10'	0.43	0.56	0.56	0.14	-	_	-		55
				12	15'	0.54	0.65	0.64	0.14	-	-	-		50
				12	20'	0.70	0.86	0.83	0.14	-	-	-		50
	11.5	11.5	11	8 to	25'	0.83	1.07	1.03	0.14	-	_	-		50
	13	13	11	12	30'	0.85	1.22	1.19	0.14	-	-	-		50
				4	0.33' - <2'	0.42	0.67	0.57	0.27	0.27	0.39	0.43	5	-
				4	2' - <3'	0.43	0.67	0.57	0.14	-	-	-	te	62
				+0	3' - <5'	0.39	0.63	0.56	0.14	-	-	-	Note	62
11' x 8'	11	11	11	to	5' - 10'	0.43	0.60	0.61	0.14	-	_	_	al al	55
				12	15'	0.54	0.72	0.71	0.14	-	-	-	ıer	50
				12	20'	0.70	0.94	0.92	0.14	-	-	-	General	50
	11.5	11.5	11	8 to	25'	0.82	1.16	1.13	0.14	-	-	-	See (	50
	13	13	11	12	30'	0.86	1.32	1.30	0.14	-	-	-	Se	50
				4	0.33' - <2'	0.44	0.71	0.62	0.27	0.27	0.38	0.44		_
				4	2' - <3'	0.44	0.71	0.62	0.14	-	-	-		75
				+-	3' - <5'	0.41	0.67	0.61	0.14	-	-	_		69
11' × 10'	11	11	11	to	5' - 10'	0.47	0.64	0.66	0.14	-	-	_		62
	_	_		12	15'	0.59	0.78	0.78	0.14	_	_	_		55
				12	20'	0.75	1.03	1.01	0.14	_	_	_		50
	11.5	12	11	8 to	25'	0.85	1.24	1.22	0.14	-	-	_		50
	13	13.5	11	12	30'	0.91	1.40	1.39	0.14	-	-	-		50
				4	0.33' - <2'	0.45	0.72	0.64	0.27	0.27	0.39	0.45		_
				4	2' - <3'	0.45	0.72	0.64	0.18	-	-	-		86
					3' - <5'	0.42	0.69	0.63	0.18	_	_	_		75
11' × 11'	11	11	11	to	5' - 10'	0.51	0.66	0.69	0.16	_	_	_		69
^				1.0	15'	0.63	0.81	0.82	0.14	_	_	_		55
				12	20'	0.80	1.07	1.06	0.14	_	_	_		55
	11.5	12	11	8 to	25'	0.91	1.29	1.27	0.14	_	_	_		50
	13	13.5	11	12	30'	0.99	1.44	1.44	0.14	_	_	_		50
		10.0				0.55	2.77	1.77	0.17	I	<u> </u>		<u> </u>	

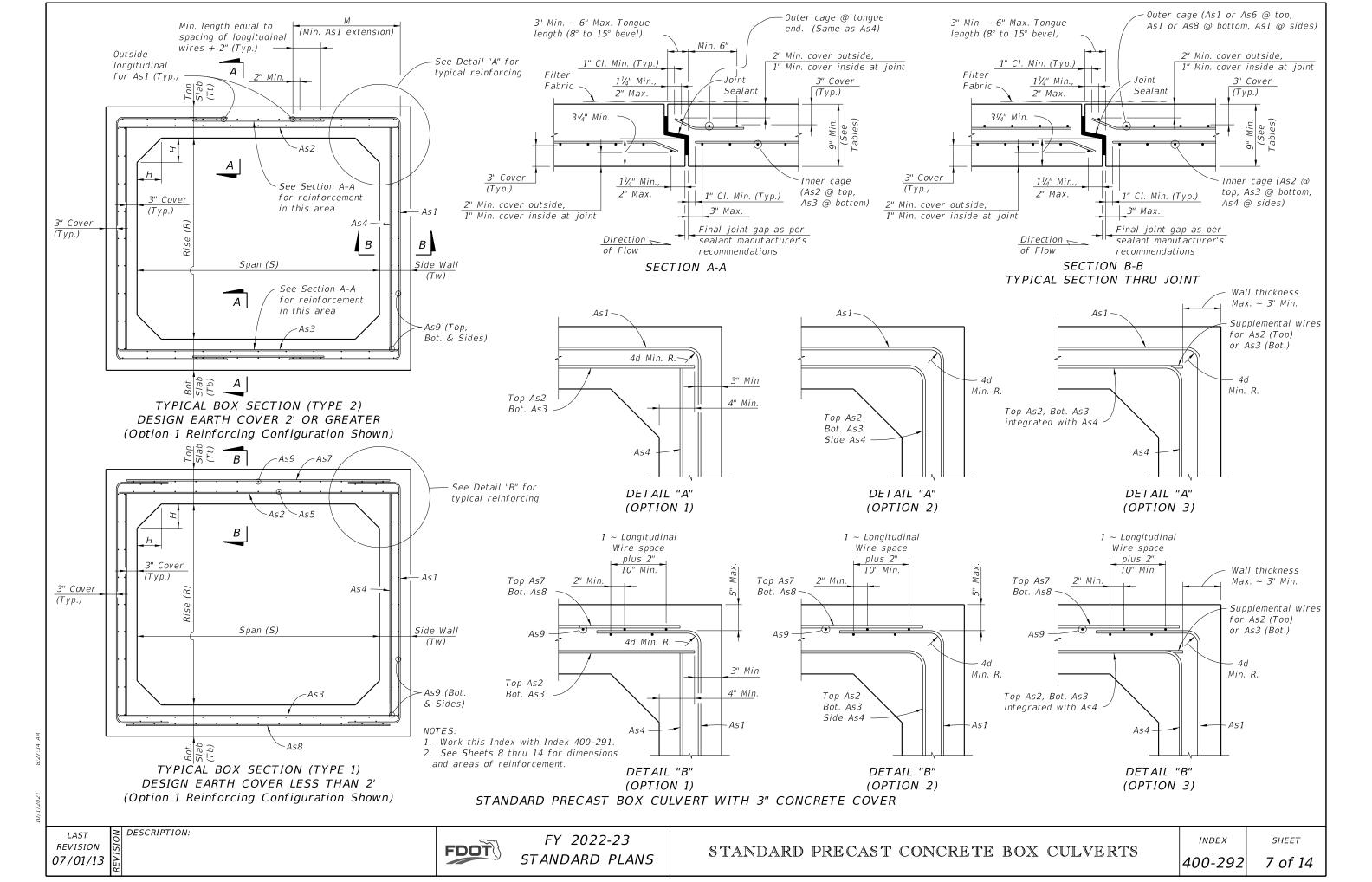
TAB	LE 8	- STA	NDAF	RD PRE	CAST BOX (	CULVE	RT D	ESIGI	V <i>S (2</i> '	" COV	′ER) -	· 12' S	PANS	5
SPAN x RISE (S) (R)	SLAE TOP	B / WAL		KNESS HAUNCH	DESIGN EARTH COVER			R	EINFOR (s	RCEMEN q. in./F		15		As1 EXT LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE				•	,	Í			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.52	0.57	0.45	0.29	0.29	0.47	0.49		_
				7	2' - <3'	0.52	0.57	0.45	0.15	-	_	-		73
				to	3' - <5'	0.50	0.54	0.45	0.15	-	-	-		66
12' x 4'	12	12	12		5' - 10'	0.50	0.52	0.52	0.15	-	-	-		66
				12	15'	0.63	0.61	0.59	0.15	-	-	-		59
					20'	0.82	0.81	0.77	0.15	-	-	-		59
	12.5	12.5	12	8 to	25'	0.99	0.99	0.95	0.15	-	-	-		59
	14	14	12	12	30'	1.03	1.15	1.11	0.15	-	-	-		59
				4	0.33' - <2'	0.47	0.62	0.51	0.29	0.29	0.42	0.46		-
					2' - <3'	0.47	0.62	0.51	0.15	-	-	-		66
				to	3' - <5'	0.45	0.60	0.51	0.15	-	-	-		59
12' x 6'	12	12	12		5' - 10'	0.47	0.59	0.59	0.15	-	-	-		59
				12	15'	0.57	0.68	0.66	0.15	-	-	-		53
					20'	0.74	0.90	0.86	0.15	-	-	-		53
	12.5	12.5	12	8 to	25'	0.88	1.11	1.06	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.27	1.24	0.15	-	-	-		53
				4	0.33' - <2'	0.44	0.67	0.56	0.29	0.29	0.40	0.44	7	-
					2' - <3'	0.44	0.67	0.56	0.15	-	-	-	Note	66
				to	3' - <5'	0.41	0.64	0.56	0.15	-	-	-		59
12' x 8'	12	12	12		5' - 10'	0.45	0.63	0.64	0.15	-	-	-	General	59
				12	15'	0.56	0.75	0.73	0.15	-	-	-	ine	53
					20'	0.72	0.98	0.95	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.85	1.20	1.16	0.15	-	-	-	See	53
	14	14.5	12	12	30'	0.89	1.38	1.35	0.15	-	-	-	S	53
				4	0.33' - <2'	0.44	0.71	0.60	0.29	0.29	0.39	0.44		_
					2' - <3'	0.44	0.71	0.60	0.15	-	-	-		73
				to	3' - <5'	0.42	0.68	0.60	0.15	-	-	-		66
12' × 10'	12	12	12		5' - 10'	0.47	0.67	0.69	0.15	-	-	-		59
				12	15'	0.59	0.81	0.81	0.15	-	-	-		53
					20'	0.75	1.06	1.04	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.87	1.30	1.26	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.47	1.45	0.15	-	-	-		53
				4	0.33' - <2'	0.46	0.74	0.64	0.29	0.29	0.40	0.46		
					2' - <3'	0.46	0.74	0.64	0.20	-	-	-		93
				to	3' - <5'	0.42	0.72	0.64	0.20	-	-	-		80
12' x 12'	12	12	12		5' - 10'	0.54	0.71	0.74	0.18	-	-	-		73
				12	15'	0.66	0.87	0.89	0.15	-	-	-		59
					20'	0.83	1.14	1.13	0.15	-	-	-		59
	12.5	13	12	8 to	25'	0.96	1.39	1.37	0.15	-	-	-		53
	14	14.5	12.5	12	30'	1.05	1.56	1.56	0.15	-	-	-		53

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

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SPAN x RISE	SLAE	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	iS		As1 EXT
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(F. )	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.22	0.24	0.22	0.22	0.22	0.22	0.22		_
					2' - <3'	0.11	0.23	0.22	0.11	_	-	_		31
				4	3' - <5'	0.11	0.22	0.22	0.11	_	-	_		31
					5' - 10'	0.11	0.22	0.22	0.11	-	-	-		31
3' x 3'	9	9	9	to	15'	0.11	0.22	0.22	0.11	-	-	-		31
					20'	0.13	0.22	0.22	0.11	-	-	_		31
				8	25'	0.16	0.22	0.22	0.11	-	-	_		31
					30'	0.19	0.24	0.25	0.11	-	-	_		31
					35'	0.22	0.28	0.29	0.11	-	-	_		31
					0.33' - <2'	0.22	0.32	0.24	0.22	0.22	0.22	0.22	5	-
				4	2' - <3'	0.17	0.31	0.24	0.11	_	-	-	Note	38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 3'	9	9	9	to	5' - 10'	0.13	0.22	0.22	0.11	-	-	-	General	38
					15'	0.17	0.22	0.22	0.11	-	-	-	ne	38
				8	20'	0.23	0.26	0.27	0.11	-	-	-		38
					25'	0.28	0.32	0.34	0.11	-	-	-	ве	38
					30'	0.33	0.39	0.40	0.11	=	-	=	S	38
					0.33' - <2'	0.22	0.34	0.26	022	0.22	0.22	0.22		_
				4	2' - <3'	0.17	0.33	0.26	0.11	-	-	-		38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 4'	9	9	9	to	5' - 10'	0.14	0.22	0.22	0.11	-	-	=		38
					15'	0.19	0.22	0.23	0.11		-	-		38
				8	20'	0.24	0.28	0.30	0.11	_	-	-		38
					25'	0.29	0.36	0.37	0.11	_	-	-		38
					30'	0.34	0.43	0.45	0.11	-	-	-		38

TABLE	9B -	STAN	DARD	PREC	AST BOX CL	ILVER	T DE.	SIGNS	5 (3"	COVE	R) - 3	3' & 4	' SPA	NS
SPAN x RISE (S) (R)	SLAE TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R		RCEMEN q. in./F		15		As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.24	0.24	0.24	0.24	0.24	0.24		-
					2' - <3'	0.12	0.24	0.24	0.24	-	-	-		31
				4	3' - <5'	0.12	0.24	0.24	0.24	-	-	-		31
					5' - 10'	0.12	0.24	0.24	0.24	-	-	-		31
3' x 3'	10	10	10	to	15'	0.12	0.24	0.24	0.24	-	-	-		31
					20'	0.12	0.24	0.24	0.24	-	-	-		31
				8	25'	0.13	0.24	0.24	0.24	-	-	-		31
					30'	0.15	0.24	0.24	0.12	-	-	-		31
					35'	0.18	0.24	0.24	0.12	-	-	-		31
					0.33' - <2'	0.24	0.26	0.24	0.24	0.24	0.24	0.24	5	-
				4	2' - <3'	0.14	0.26	0.24	0.12	-	-	-	Note	38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-	No	38
4' x 3'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-	le.	38
					15'	0.14	0.24	0.24	0.12	-	-	-	General	38
				8	20'	0.18	0.24	0.24	0.12	-	-	-	Ge.	38
					25'	0.22	0.26	0.27	0.12	-	-	-	ee ee	38
					30'	0.26	0.31	0.32	0.12	-	-	-	Se	38
					0.33' - <2'	0.24	0.28	0.24	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.14	0.28	0.24	0.12	-	-	-		38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-		38
4' x 4'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-		-		38
					15'	0.15	0.24	0.24	0.12	-	-	-		38
				8	20'	0.19	0.24	0.24	0.12	-	-	-		38
					25'	0.23	0.28	0.30	0.12	-	-	-		38
					30'	0.27	0.34	0.35	0.12	-	-	-		38

- See Sheet 2 for General Notes.
   See Sheet 7 for Reinforcing Details and dimension locations.
   See Sheet 14 for WWR Bending Diagrams.

LAST	NC	DESCRIPTION:
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,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	IUA -	31 AN	IDAKL	PREC	AST BOX CU	JLVEF	KI DE	SIGN.	5 (3"	COVE	(R) -	5' & (	5' SP	4 <i>NS</i>
AN x RISE		/ WAL			DESIGN			R	EINFOR			15		As1 EX
) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			1					1	(M)
(FL.)	(in.)	(in.)	(in.)	(in.)	TUP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.27	0.39	0.37	0.22	0.22	0.22	0.27		-
				4	2' - <3'	0.26	0.39	0.37	0.11	-	-	_		45
					3' - <5'	0.19	0.24	0.25	0.11	_	-	-	]	36
5' x 3'	9	9	9	to	5' - 10'	0.20	0.22	0.22	0.11	_	_	_	1	36
					15'	0.28	0.28	0.30	0.11	_	_	_		35
				8	20'	0.37	0.38	0.39	0.11	_	_	_	1	35
					25'	0.45	0.48	0.49	0.11	_	_	_	-	35
					30'	0.54	0.58	0.59	0.11	_	_	_		35
					0.33' - <2'	0.26	0.42	0.39	0.22	0.22	0.22	0.26	-	
													-	
				4	2' - <3'	0.26	0.42	0.39	0.11	-	-	-		45
					3' - <5'	0.19	0.26	0.27	0.11	-	-	-		45
5' x 4'	9	9	9	to	5' - 10'	0.20	0.22	0.23	0.11	-	-	-		36
					15'	0.27	0.31	0.33	0.11	-	-	-	ļ	35
				8	20'	0.36	0.42	0.43	0.11	-	-	-		35
					25'	0.44	0.52	0.54	0.11	-	-	-	]	35
					30'	0.53	0.63	0.65	0.11	-	-	-	]	35
					0.33' - <2'	0.27	0.44	0.42	0.22	0.22	0.22	0.27		-
				4	2' - <3'	0.27	0.44	0.42	0.11	-	-	_	]	45
					3' - <5'	0.20	0.27	0.28	0.11	_	-	_	1	45
5' x 5'	9	9	9	to	5' - 10'	0.22	0.23	0.26	0.11	-	_	_	1	45
				10	15'	0.30	0.34	0.36	0.11	_	_	_		36
				8	20'	0.38	0.45	0.47	0.11	_	_	_	-	35
				0	25'	0.47	0.56	0.59	0.11	_	_	_	!	35
					30'	0.55	0.68	0.71	0.11	_			-	35
												-		-
					0.33' - <2'	0.34	0.47	0.42	0.22	0.22	0.25	0.34	7.	
				4	2' - <3'	0.34	0.47	0.42	0.11	-	-	-	Note	43
					3' - <5'	0.27	0.31	0.32	0.11	-	-	-	>	39
6' x 3'	9	9	9	to	5' - 10'	0.29	0.26	0.28	0.11	-	-	-	General	39
					15'	0.42	0.39	0.40	0.11	-	-	-	ne	38
				12	20'	0.55	0.52	0.53	0.11	-	-	_	<i>Ge</i>	38
					25'	0.68	0.66	0.67	0.11	-	-	-	See	38
					30'	0.82	0.81	0.82	0.11	-	-	-	Se	38
					0.33' - <2'	0.33	0.50	0.46	0.22	0.22	0.23	0.33	1	_
				4	2' - <3'	0.33	0.50	0.46	0.11	_	_	_		43
				7	3' - <5'	0.27	0.33	0.35	0.11	_	_	_		39
6' x 4'	9	9	9	4	5' - 10'		0.29	0.31						39
U / 4	9	) 9	9	to	15'					_		_	1	38
						0.40	0.43	0.45	0.11				-	
				12	20'	0.52	0.57	0.59	0.11	-	-	-	-	38
					25'	0.65	0.73	0.74	0.11	-	-	-	-	38
					30'	0.78	0.88	0.90	0.11	-	-	-		38
					0.33' - <2'	0.33	0.52	0.49	0.22	0.22	0.23	0.33		_
				4	2' - <3'	0.33	0.52	0.49	0.11	-	-	_		43
					3' - <5'	0.27	0.35	0.37	0.11	-	-	-	]	43
6' x 5'	9	9	9	to	5' - 10'	0.29	0.31	0.34	0.11	-	-	-	]	39
					15'	0.41	0.46	0.49	0.11	-	-	-		38
				12	20'	0.53	0.62	0.64	0.11	-	-	-	1	38
					25'	0.66	0.78	0.80	0.11	_	_	_	1	38
					30'	0.78	0.95	0.97	0.11	_	_	_	1	38
					0.33' - <2'	0.34	0.55	0.51	0.22	0.22	0.24	0.34	1	-
				_	2' - <3'		0.53	0.51	0.22			0.54	-	
				4		0.34				-	-		-	52
					3' - <5'	0.29	0.37	0.39	0.11	-	-	-	-	52
6' x 6'	9	9	9	to	5' - 10'	0.32	0.34	0.37	0.11	-	-	_		43
					15'	0.44	0.50	0.53	0.11	-	-	-		39
				12	20'	0.57	0.66	0.70	0.11	-	-	-	]	39
					<i>25</i> ′	0.70	0.84	0.87	0.11	-	-	_	]	38
					30'	0.83	1.02	1.05	0.11	-	-	-		38

					AST BOX CU	JEVET	II DL						) ),	
SPAN x RISE			L THIC		DESIGN			R	EINFOF			15		As1 EX
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB				T	T	T	T		(M) (in.)
(1)	(111.)	(111.)	(111.)	(111.)		As 1	A52	As3	A54	As5	As7	A58	As9	(111.)
					0.33' - <2'	0.24	0.33	0.32	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.22	0.33	0.32	0.12	-	-	-		45
E! v D!	10	10	10		3' - <5' 5' - 10'	0.16	0.24 0.24	0.24	0.12	-	-	-		36
5' x 3'	10	10	10	to	15'	0.16	0.24	0.24	0.12	_	_	_		35
				12	20'	0.29	0.30	0.24	0.12	_	_			35
				12	25'	0.36	0.38	0.39	0.12	_	_	_		35
					30'	0.43	0.46	0.47	0.12	_	_	_		35
					0.33' - <2'	0.24	0.35	0.34	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.22	0.35	0.34	0.12	-	_	_		45
				,	3' - <5'	0.15	0.24	0.24	0.12	-	-	-		45
5' x 4'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-		36
					15'	0.22	0.25	0.27	0.12	-	-	-		35
				12	20'	0.29	0.33	0.34	0.12	-	-	-		35
					25'	0.36	0.41	0.43	0.12	-	-	-		35
					30'	0.42	0.50	0.51	0.12	-	-	-		35
					0.33' - <2'	0.24	0.37	0.36	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.21	0.37	0.36	0.12	-	-	-		45
		1			3' - <5'	0.16	0.24	0.25	0.12	-	-	-		45
5' x 5'	10	10	10	to	5' - 10'	0.17	0.24	0.24	0.12	-	-	-		45
					15' 20'	0.24	0.27	0.29	0.12	-	_	-		36
				12	25'	0.30	0.36 0.44	0.38	0.12	_	_	_		35 35
					30'	0.37	0.44	0.47	0.12	_	_	_		35
					0.33' - <2'	0.28	0.40	0.36	0.12	0.24	0.24	0.28	5	
				4	2' - <3'	0.28	0.40	0.36	0.12	-	-	-		43
				"	3' - <5'	0.22	0.26	0.28	0.12	_	_	_	Note	39
6' x 3'	10	10	10	to	5' - 10'	0.24	0.24	0.24	0.12	_	_	_		39
					15'	0.34	0.31	0.32	0.12	-	_	_	General	38
				12	20'	0.44	0.41	0.42	0.12	-	-	-	Ge!	38
					25'	0.54	0.52	0.53	0.12	-	-	-	See	38
					30'	0.64	0.63	0.64	0.12	-	-	-	Se	38
					0.33' - <2'	0.27	0.42	0.39	0.24	0.24	0.24	0.27		-
				4	2' - <3'	0.27	0.42	0.39	0.12	-	-	-		43
					3' - <5'	0.21	0.28	0.30	0.12	-	-	-		39
6' x 4'	10	10	10	to	5' - 10'	0.23	0.24	0.25	0.12	-	-	-		39
					15'	0.32	0.34	0.35	0.12	-	-	-		38
				12	20'	0.42	0.45	0.47	0.12	-	-	-		38
					25'	0.51	0.56	0.58	0.12	-	-	-		38
					30'	0.61	0.68	0.70	0.12	- 0.24	- 0.24	0.26		38
				_	0.33' - <2'	0.26	0.44	0.42	0.24	0.24	0.24	0.26		12
				4	2' - <3' 3' - <5'	0.26	0.44	0.42	0.12	-	-	-		43
6' x 5'	10	10	10	+ 0	5' - 10'	0.22	0.30	0.33	0.12	_	_	_		39
0 / 0	10	10	10	to	15'	0.24	0.25	0.27	0.12	_	_	_		38
				12	20'	0.33	0.48	0.53	0.12	_	_	_		38
				12	25'	0.52	0.40	0.63	0.12	_	_	_		38
					30'	0.61	0.74	0.76	0.12	-	_	_		38
					0.33' - <2'	0.27	0.46	0.44	0.24	0.24	0.24	0.27		-
				4	2' - <3'	0.27	0.46	0.44	0.12	-	-	-		52
				7	3' - <5'	0.23	0.31	0.34	0.12	_	-	-		52
6' x 6'	10	10	10	to	5' - 10'	0.25	0.27	0.30	0.12	_	-	_		43
-					15'	0.35	0.39	0.42	0.12	-	-	-		39
				12	20'	0.45	0.52	0.55	0.12	-	-	-		39
					25'	0.54	0.65	0.68	0.12	-	-	_		38
					30'	0.64	0.78	0.81	0.12	_	_	_		38

LAST REVISION 07/01/13

≥ DESCRIPTION:

FDOT

TABL	E 11A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPAN:	5
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(=, )	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.42	0.58	0.52	0.22	0.22	0.31	0.42		-
				4	2' - <3'	0.42	0.58	0.51	0.11	-	-	-		43
					3' - <5'	0.36	0.41	0.44	0.11	-	-	-		43
7' x 4'	9	9	9	to	5' - 10'	0.39	0.40	0.39	0.11	-	-	-		43
					15'	0.56	0.56	0.58	0.11	-	-	-		41
				12	20'	0.74	0.76	0.77	0.11	-	-	-		41
					25'	0.92	0.97	0.97	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.09	1.18	1.10	0.11	-	-	-		41
					0.33' - <2'	0.41	0.61	0.55	0.22	0.23	0.30	0.41		-
				4	2' - <3'	0.41	0.61	0.55	0.11	-	-	-		47
					3' - <5'	0.37	0.43	0.47	0.11	-	-	-		43
7' x 5'	9	9	9	to	5' - 10'	0.39	0.41	0.43	0.11	-	-	-		43
					15'	0.56	0.61	0.63	0.11	-	-	-	5	41
				12	20'	0.73	0.82	0.83	0.11	-	-	-	te	41
					25'	0.90	1.04	1.06	0.11	-	-	-	No	41
	9	9.5	9	7 to 12	30'	1.06	1.26	1.19	0.11	-	-	-	General Note	41
					0.33' - <2'	0.42	0.63	0.58	0.22	0.24	0.30	0.42	neı	-
				4	2' - <3'	0.42	0.63	0.58	0.11	-	-	-	Ge	59
					3' - <5'	0.38	0.45	0.50	0.11	-	-	-	See	47
7' x 6'	9	9	9	to	5' - 10'	0.41	0.44	0.47	0.11	-	-	-	S	43
					15'	0.57	0.65	0.68	0.11	-	-	-		41
				12	20'	0.75	0.87	0.90	0.11	-	-	-		41
					25'	0.93	1.11	1.13	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.07	1.35	1.27	0.11	-	-	-		41
					0.33' - <2'	0.44	0.66	0.61	0.22	0.25	0.31	0.44	]	-
				4	2' - <3'	0.44	0.65	0.61	0.11	-	-	_	]	59
					3' - <5'	0.41	0.47	0.52	0.11	-	-	_		59
7' x 7'	9	9	9	to	5' - 10'	0.44	0.47	0.52	0.11	-	-	-		47
					15'	0.62	0.69	0.74	0.11	-	-	_	]	43
				12	20'	0.80	0.93	0.97	0.11	_	-	_		43
					25'	0.99	1.18	1.22	0.11	-	-	-		43
	9	9.5	9	7 to 12	30'	1.12	1.43	1.36	0.11	_	-	-		41

TABI	LE 11B	B - ST	ANDA	RD PRI	ECAST BOX	CULV	ERT	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPANS	5
SPAN x RISE	SLAB	7 WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	вот.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0V E									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.33	0.49	0.44	0.24	0.24	0.24	0.33		-
				4	2' - <3'	0.33	0.49	0.44	0.12	-	-	-		43
					3' - <5'	0.29	0.35	0.38	0.12	-	-	-		43
7' x 4'	10	10	10	to	5' - 10'	0.31	0.30	0.31	0.12	-	-	-		43
					15'	0.44	0.44	0.45	0.12	-	-	-		41
				12	20'	0.58	0.59	0.60	0.12	-	_	-		41
					25'	0.71	0.74	0.75	0.12	-	-	-		41
					30'	0.85	0.91	0.91	0.12	-	-	-		41
					0.33' - <2'	0.32	0.51	0.47	0.24	0.24	0.24	0.32		-
				4	2' - <3'	0.32	0.51	0.47	0.12	-	-	-		47
					3' - <5'	0.29	0.37	0.41	0.12	-	-	-		43
7' x 5'	10	10	10	to	5' - 10'	0.31	0.32	0.35	0.12	-	-	-		43
					15'	0.44	0.47	0.50	0.12	-	-	-	5	41
				12	20'	0.57	0.63	0.65	0.12	-	-	-	ıte	41
					25'	0.70	0.80	0.82	0.12	-	-	-	Note	41
					30'	0.84	0.97	0.99	0.12	-	-	-	General	41
					0.33' - <2'	0.33	0.53	0.50	0.24	0.24	0.24	0.33	neı	-
				4	2' - <3'	0.33	0.53	0.50	0.12	-	-	-	Ge.	59
					3' - <5'	0.30	0.38	0.43	0.12	-	-	-	өө	47
7' x 6'	10	10	10	to	5' - 10'	0.33	0.35	0.38	0.12	-	-	-	56	43
					15'	0.45	0.51	0.54	0.12	-	-	-		41
				12	20'	0.58	0.68	0.70	0.12	-	-	-		41
					25'	0.72	0.85	0.88	0.12	-	-	-		41
					30'	0.85	1.04	1.06	0.12	-	-	-		41
					0.33' - <2'	0.35	0.55	0.52	0.24	0.24	0.24	0.35		-
				4	2' - <3'	0.35	0.55	0.52	0.12	-	-	-		59
					3' - <5'	0.32	0.40	0.46	0.12	-	-	-		59
7' x 7'	10	10	10	to	5' - 10'	0.35	0.37	0.41	0.12	-	-	-		47
					15'	0.48	0.54	0.58	0.12	-	-	-		43
				12	20'	0.62	0.72	0.76	0.12	-	_	-		43
					25'	0.76	0.90	0.94	0.12	-	-	-		43
					30'	0.90	1.10	1.13	0.12	-	-	-		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
   See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

≥ DESCRIPTION:

FDOT

(5) (7) (7) (7) (7) (7) (7) (70) (70) (70)	TABL	E 124	\ - ST	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (	3" CC	VER)	- 8' .	SPAN	S
(FL) (7T) (7T) (7T) (1H) (1H) (1H) TOP SLAB	SPAN x RISE		/ WAL	L THIC	KNESS				R				15		
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	(S) (R)	-		_						(5	q. in./F	t.)			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	(51.)														
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					1	0.33' - <2'	0.52	0.66	0.57	0.22	0.24	0.42	0.52		-
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	2' - <3'	0.52	0.66	0.57	0.11	-	-	-		50
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					to	3' - <5'	0.48	0.49	0.52	0.11	-	-	-		50
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	8' x 4'	9	9	9	ισ	5' - 10'	0.52	0.48	0.49	0.11	-	-	-		45
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					12	15'	0.75	0.72	0.72	0.11	-	-	-		41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					12	20'	1.00	0.98	0.97	0.11	-	-	-		41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		9	9.5	9	8 to	25'	1.25	1.24	1.14	0.11	-	-	-		41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		10	10.5	9	12	30'	1.31	1.29	1.21	0.11	-	-	-		41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					1	0.33' - <2'	0.51	0.69	0.60	0.22	0.25	0.40	0.51		_
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	2' - <3'	0.51	0.69	0.60	0.11	_	_			50
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					+ 0	3' - <5'	0.46	0.52	0.56	0.11	-	-	-		45
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	8' x 5'	9	9	9	10	5' - 10'	0.51	0.51	0.53	0.11	-	-	_		45
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					1 7	15'	0.74	0.77	0.78	0.11	-	-	-		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					12	20'	0.97	1.05	1.05	0.11	-	-	-		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		9	9.5	9	8 to	25'	1.20	1.33	1.23	0.11	-	-	_		41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		10	10.5	9	12	30'	1.26	1.38	1.30	0.11	-	-	_		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	0.33' - <2'	0.51	0.72	0.64	0.22	0.26	0.39	0.51	5	-
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	2' - <3'	0.51	0.72	0.64	0.11	-	-	-	te	50
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$						3' - <5'	0.47	0.55	0.59	0.11	-	-	-	No	50
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	8' x 6'	9	9	9	τo	5' - 10'	0.52	0.55	0.58	0.11	-	-	-		45
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					1.7	15'	0.74	0.83	0.85	0.11	-	-	-	Jer.	41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					12	20'	0.97	1.12	1.13	0.11	-	-	-	96/	41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		9	9.5	9	8 to	25'	1.18	1.42	1.32	0.11	-	-	-	e e	41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		10	10.5	9	12	30'	1.26	1.46	1.39	0.11	-	-	-	Se	41
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	0.33' - <2'	0.52	0.74	0.67	0.22	0.26	0.40	0.52		_
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	2' - <3'	0.52	0.74	0.67	0.11	-	-	_		55
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					<b>+</b> -		0.49	0.57	0.62	0.11	-	-	_		55
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	8' x 7'	9	9	9	10	5' - 10'	0.55	0.59	0.63	0.11	-	-	-		50
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					1 2	15'	0.77	0.88	0.91	0.11	-	-	-		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					12	20'	1.01	1.19	1.21	0.11	-	-	_		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		9	9.5	9	8 to	25'	1.21	1.51	1.41	0.11	-	-	-		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		10	10.5	9	12	30'	1.31	1.53	1.47	0.11	-	-	-		41
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	0.33' - <2'	0.55	0.77	0.70	0.22	0.27	0.41	0.55		-
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$					4	2' - <3'	0.55	0.77	0.70	0.13	-	_	-		65
8' x 8' 9 9 9 9 0.60 0.60 0.63 0.68 0.11 55 15' 0.83 0.93 0.98 0.11 55 45 20' 1.08 1.26 1.29 0.11 45 45 45 9 9.5 9 8 to 25' 1.28 1.59 1.50 0.11 41					4 -	3' - <5'	0.53	0.59	0.64	0.12	-	-	-		65
9 9.5 9 8 to 25' 1.28 1.59 1.50 0.11 41	8' x 8'	9	9	9	ŧο	5' - 10'	0.60	0.63	0.68	0.11	-	-	-		55
9 9.5 9 8 to 25' 1.28 1.59 1.50 0.11 41					1.3	15'	0.83	0.93	0.98	0.11	-	-	-		45
					12	20'		1.26	1.29	0.11	-	_	_		45
		9	9.5	9	8 to						-	-	_		
		10	10.5		12		1.41		1.55		_	_	_		

SPAN x RISE	SLAE	3 / WAL	L THIC	KNESS	DESIGN	CULV		R	EINFOF	RCEMEN	T AREA	15		As1 EX
(S) (R)	TOP	ВОТ.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.42	0.56	0.49	0.24	0.24	0.32	0.41		_
				4	2' - <3'	0.42	0.56	0.49	0.12	-	-	-		50
					3' - <5'	0.38	0.42	0.46	0.12	-	-	-		50
8' x 4'	10	10	10	to	5' - 10'	0.41	0.38	0.39	0.12	-	-	-		45
					15'	0.59	0.56	0.57	0.12	-	-	-		41
				12	20'	0.78	0.75	0.76	0.12	-	-	-		41
					25'	0.97	0.96	0.96	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.15	1.16	1.10	0.12	-	-	-		41
					0.33' - <2'	0.40	0.58	0.52	0.24	.034	0.31	0.40		
				4	2' - <3'	0.40	0.58	0.52	0.12	-	-	-		50
o. =:					3' - <5'	0.37	0.45	0.48	0.12	-	-	-		45
8' x 5'	10	10	10	to	5' - 10'	0.41	0.41	0.43	0.12	-	-	-		45
					15'	0.58	0.60	0.62	0.12	-	-	-		41
				12	20'	0.76	0.81	0.81	0.12	-	-	-		41
					25'	0.94	1.03	1.03	0.12	-	_	-		41
	10	10.5	10	8 to 12	30'	1.10	1.24	1.24	0.12	-	-	-		41
					0.33' - <2'	0.40	0.60	0.55	0.24	0.24	0.30	0.40	5 5	-
				4	2' - <3'	0.40	0.60	0.55	0.12	-	-	-	Note	50
01 61	1.0	1.0	1.0		3' - <5'	0.37	0.47	0.51	0.12	-	-	-		50
8' x 6'	10	10	10	to	5' - 10' 15'	0.42	0.43	0.46	0.12	-	=	-	General	45
				1.2	20'	0.58	0.64	0.67 0.88	0.12	-	_	_	ene	41
				12	25'	0.76	1.09	1.11	0.12	_	_	_		41
	10	10.5	10	8 to 12	30'	1.09	1.32	1.26	0.12	_	_	_	See	41
	10	10.5	10	0 10 12	0.33' - <2'	0.41	0.63	0.58	0.12	0.24	0.30	0.41	·	-
				4	2' - <3'	0.41	0.63	0.58	0.12	-	-	-		55
				4	3' - <5'	0.39	0.49	0.53	0.12	_	_	_		55
8' x 7'	10	10	10	to	5' - 10'	0.44	0.46	0.50	0.12	_	_	_		50
<i>O</i> , ,			10	10	15'	0.61	0.68	0.72	0.12	_	_	_		45
				12	20'	0.78	0.91	0.94	0.12	_	_	_		41
				12	25'	0.97	1.16	1.18	0.12	_	_	-		41
	10	10.5	10	8 to 12	30'	1.11	1.40	1.34	0.12	-	-	-		41
					0.33' - <2'	0.44	0.64	0.60	0.24	0.24	0.31	0.44		_
				4	2' - <3'	0.44	0.64	0.60	0.12	-	-	-		65
				_ ′	3' - <5'	0.42	0.51	0.56	0.12	-	_	_		65
8' x 8'	10	10	10	to	5' - 10'	0.47	0.50	0.55	0.12	-	-	-		55
					15'	0.65	0.72	0.77	0.12	-	_	_		45
				12	20'	0.84	0.96	1.01	0.12	_	_	_		45
					25'	1.03	1.22	1.26	0.12	-	_	_		41
	10	10.5	10	8 to 12	30'	1.16	1.47	1.42	0.12	_	_	_		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
   See Sheet 14 for WWR Bending Diagrams.

LAST REVISION 07/01/13

≥ DESCRIPTION:

TABLI	E 13A	- ST	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (.	3" CC	VER)	- 9' 5	SPAN	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	ıS		As1 EXT.
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
4	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.62	0.78	0.65	0.22	0.26	0.52	0.61		-
				4	2' - <3'	0.62	0.78	0.65	0.11	-	-	-		54
				to	3' - <5'	0.58	0.63	0.61	0.11	-	-	-		49
9' x 5'	9	9	9	12	5' - 10'	0.65	0.63	0.64	0.11	-	-	-		49
					15'	0.95	0.96	0.95	0.11	-	-	-		44
	9	9	9	8	20'	1.26	1.32	1.28	0.11	-	-	-		44
	10	10.5	9	to	25'	1.39	1.41	1.32	0.11	-	-	-		44
	11	11.5	9	12	30'	1.46	1.50	1.42	0.11	-	-	-		44
					0.33' - <2'	0.60	0.81	0.69	0.22	0.27	0.51	0.60		-
				4	2' - <3'	0.60	0.81	0.69	0.11	-	-	-		54
				to	3' - <5'	0.56	0.66	0.65	0.11	-	_	_		49
9' x 6'	9	9	9	12	5' - 10'	0.65	0.68	0.69	0.11	-	-	-		49
					15'	0.94	1.03	1.02	0.11	-	-	-		44
	9	9	9	8	20'	1.25	1.40	1.38	0.11	-	-	-		44
l L	10	10.5	9	to	25'	1.37	1.49	1.40	0.11	1	1	1		44
	11	11.5	9	12	30'	1.44	1.58	1.50	0.11	ı	ı	1		44
					0.33' - <2'	0.61	0.84	0.72	0.22	0.28	0.51	0.61	5	-
				4	2' - <3'	0.61	0.83	0.72	0.11	1	1	ı	Note	59
				to	3' - <5'	0.58	0.69	0.68	0.11	-	-	1		54
9' x 7'	9	9	9	12	5' - 10'	0.67	0.73	0.75	0.11	1	ı	1	General	49
					15'	0.96	1.09	1.10	0.11	1	-	1	neı	44
	9	9	9	8	20'	1.27	1.49	1.47	0.11	1	-	1	<i>Ge</i>	44
	10	10.5	9	to	25'	1.38	1.57	1.48	0.11	-	-	-	See	44
	11	11.5	9	12	30'	1.49	1.70	1.58	0.11	-	_	-	Sé	44
	9	9.5	9		0.33' - <2'	0.60	0.85	0.73	0.22	0.29	0.52	0.53		-
				4	2' - <3'	0.64	0.86	0.76	0.12	-	-	1		59
				to	3' - <5'	0.62	0.72	0.72	0.11	-	-	1		59
9' x 8'	9	9	9	12	5' - 10'	0.71	0.77	0.81	0.11	-	-	-		54
					15'	1.01	1.16	1.17	0.11	-	_	1		44
	9	9.5	9	8	20'	1.27	1.56	1.45	0.11	ı	ı	1		44
	10	10.5	9	to	25'	1.45	1.65	1.57	0.11	-	-	1		44
	11	11.5	9	12	30'	1.59	1.72	1.66	0.11	-	-	1		44
	9	9.5	9		0.33' - <2'	0.68	0.88	0.76	0.22	0.29	0.55	0.57		-
				4	2' - <3'	0.68	0.88	0.78	0.18	-	_	-		72
				to	3' - <5'	0.68	0.75	0.78	0.18	-	_	-		72
9' x 9'	9	9	9	12	5' - 10'	0.79	0.82	0.88	0.17	-	-	1		59
					15'	1.11	1.22	1.26	0.13	-	-	1		49
	9	9.5	9	8	20'	1.37	1.64	1.54	0.13	-	-	-		49
	10	10.5	9	to	25'	1.56	1.73	1.65	0.13	-	-	-		44
. ⊢	11	11.5	9.5	12	30'	1.56	1.73	1.68	0.12					44

TABL	.E 13E	3 - ST	ANDA	ARD PR	ECAST BOX	( CULVERT DESIGNS (3" COVER) - 9' SPANS									
SPAN x RISE	SLAE	/ WAL			DESIGN	REINFORCEMENT AREAS								As1 EXT.	
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH	
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB				1		1			(M)	
(ГС.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	A59	(in.)	
				4	0.33' - <2'	0.49	0.65	0.57	0.24	0.24	0.40	0.48		-	
				4	2' - <3'	0.49	0.65	0.57	0.12	-	-	-		54	
	10	10	10	to	3' - <5'	0.46	0.54	0.53	0.12	-	-	-		49	
9' x 5'	10	10	10	10	5' - 10'	0.52	0.50	0.51	0.12	-	-	-		49	
				12	15'	0.75	0.74	0.75	0.12	-	-	-		44	
					20'	0.98	1.01	1.00	0.12	-	-	-		44	
	10	10.5	10	8 to	25'	1.21	1.27	1.19	0.12	-	-	-		44	
	11	11.5	10	12	30'	1.30	1.36	1.30	0.12	-	-	-		44	
				4	0.33' - <2'	0.48	0.68	0.60	0.24	0.24	0.39	0.48		-	
				4	2' - <3'	0.48	0.68	0.60	0.12	-	-	-		54	
	10	10	10	+0	3' - <5'	0.45	0.57	0.56	0.12	-	-	-		49	
9' x 6'	10	10	10	to	5' - 10'	0.52	0.53	0.56	0.12	-	-	-		49	
				12	15'	0.74	0.79	0.81	0.12	-	-	-		44	
					20'	0.97	1.07	1.07	0.12	-	-	-		44	
	10	10.5	10	8 to	25'	1.18	1.35	1.28	0.12	-	-	-		44	
	11	11.5	10	12	30'	1.27	1.44	1.38	0.12	-	-	-		44	
					0.33' - <2'	0.49	0.70	0.63	0.24	0.24	0.39	0.49	5	-	
				4	2' - <3'	0.49	0.70	0.63	0.12	-	-	-	)te	59	
	1.0	1.0	1.0		3' - <5'	0.46	0.59	0.59	0.12	-	-	-	N	54	
9' x 7'	10	10	10	to	5' - 10'	0.54	0.57	0.60	0.12	-	-	-	General Note	49	
				1.2	15'	0.75	0.84	0.86	0.12	_	-	-	neı	44	
				12	20'	0.98	1.13	1.14	0.12	-	-	-	Ge	44	
	10	10.5	10	8 to	25'	1.18	1.43	1.36	0.12	-	-	-	See	44	
	11	11.5	10	12	30'	1.28	1.52	1.46	0.12	-	-	-	Š	44	
					0.33' - <2'	0.51	0.72	0.65	0.24	0.24	0.39	0.51		-	
				4	2' - <3'	0.51	0.72	0.65	0.12	-	-	-		59	
					3' - <5'	0.49	0.61	0.62	0.12	-	-	-		59	
9' x 8'	10	10	10	to	5' - 10'	0.57	0.60	0.65	0.12	-	-	-		54	
					15'	0.79	0.89	0.92	0.12	-	_	-		44	
				12	20'	1.02	1.20	1.22	0.12	-	-	-		44	
	10	10.5	10	8 to	25'	1.21	1.50	1.44	0.12	_	-	-		44	
	11	11.5	10	12	30'	1.33	1.59	1.54	0.12	-	-	-		44	
					0.33' - <2'	0.54	0.74	0.68	0.24	0.24	0.41	0.54		-	
				4	2' - <3'	0.54	0.74	0.68	0.15	-	-	-		72	
				1 .	3' - <5'	0.53	0.63	0.64	0.13	-	-	-		72	
9' x 9'	10	10	10	to	5' - 10'	0.62	0.64	0.70	0.12	-	-	-	1	59	
					15'	0.85	0.94	0.99	0.12	-	-	-	1	49	
				12	20'	1.09	1.26	1.29	0.12	-	-	-	1	49	
İ	10	10.5	10	8 to	25'	1.28	1.56	1.52	0.12	-	-	-		44	

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
   See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: REVISION 07/01/13

SPAN x RISE	SLAB	/ WAL			DESIGN REINFORCEMENT AREAS									As1 EXT
(S) (R)	TOP	BOT.			EARTH COVER				(5	q. in./F	t.)			LENGT
(Ft.)	(Tt)	(Tb)	(Tw)	(H) (in.)	ABOVE TOP SLAB				1					(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(1n.)		As1	A52	As3	As4	As5	A57	A58	As9	(111.)
				4	0.33' - <2'	0.60	0.73	0.61	0.24	0.24	0.50	0.57		-
				4 to	2' - <3' 3' - <5'	0.60 0.57	0.73	0.61 0.58	0.12	-	-	-		58 53
10' x 5'	10	10	10	12	5' - 10'	0.57	0.60	0.58	0.12	_	-	-		52
10 x 3	10	10	10	12	15'	0.03	0.90	0.89	0.12	_	_	_		47
	10	10	10	8	20'	1.24	1.23	1.19	0.12	_	_	_		47
	11	11.5	10	to	25'	1.39	1.37	1.28	0.12	_	_	_		47
	12.5	12.5	10	12	30'	1.38	1.43	1.41	0.12	_	-	-		47
					0.33' - <2'	0.58	0.75	0.64	0.24	0.24	0.48	0.56		_
				4	2' - <3'	0.58	0.75	0.64	0.12	-	-	_		58
				to	3' - <5'	0.56	0.67	0.62	0.12	-	-	-		52
10' x 6'	10	10	10	12	5' - 10'	0.64	0.64	0.65	0.12	-	-	-		52
					15'	0.92	0.96	0.95	0.12	-	-	-		47
	10	10	10	8	20'	1.21	1.31	1.27	0.12	-	-	-		47
	11	11.5	10	to	25'	1.35	1.44	1.36	0.12	-	-	-		47
	12.5	12.5	10	12	30'	1.35	1.51	1.49	0.12	-	-	-		47
					0.33' - <2'	0.57	0.78	0.67	0.24	0.24	0.48	0.57		-
				4	2' - <3'	0.57	0.78	0.67	0.12	-	-	-		58
10' x 7'				to	3' - <5'	0.58	0.70	0.65	0.12	-	-	-		58
	10	10	10	12	5' - 10'	0.65	0.68	0.70	0.12	-	-	-		52
					15'	0.92	1.02	1.02	0.12	-	-	-	5	47
	10	10	10	8	20'	1.21	1.38	1.35	0.12	-	-	-	Note	47
	11	11.5	10	to 12	25' 30'	1.33	1.52	1.44	0.12	-	-	-		47
	12.5	12.5	10	12	0.33' - <2'	1.38	1.58 0.80	1.57 0.70	0.12	- 0.20	- 0.40	- 0.58	General	47
				4	0.33 - <2 2' - <3'	0.58 0.58	0.80	0.70	0.24	0.26	0.48	0.58	en	64
				to	3' - <5'	0.60	0.72	0.68	0.12	_	_	_		58
10' × 8'	10	10	10	12	5' - 10'	0.67	0.72	0.75	0.12	_	_	_	See	52
10 % C		.0			15'	0.95	1.08	1.08	0.12	_	_	_		47
	10	10	10	8	20'	1.24	1.45	1.44	0.12	_	_	_		47
	11	11.5	10	to	25'	1.36	1.59	1.52	0.12	-	-	-		47
	12.5	12.5	10	12	30'	1.45	1.64	1.64	0.12	-	-	-		47
					0.33' - <2'	0.61	0.82	0.73	0.24	0.26	0.50	0.61		_
				4	2' - <3'	0.61	0.82	0.73	0.14	-	-	-		70
				to	3' - <5'	0.64	0.75	0.73	0.13	-	-	-		64
10' x 9'	10	10	10	12	5' - 10'	0.72	0.77	0.80	0.12	-	-	-		58
					15'	1.00	1.13	1.15	0.12	-	-	-		52
	10	10	10	8	20'	1.30	1.53	1.52	0.12	-	-	-		47
	11	11.5	10	to	25'	1.42	1.66	1.60	0.12	-	-	_		47
	12.5	12.5	10	12	30'	1.57	1.70	1.72	0.12	-	-	-		47
					0.33' - <2'	0.66	0.84	0.75	0.24	0.27	0.52	0.65		-
				4	2' - <3'	0.66	0.84	0.75	0.20	-	-	-		79
10/ 10/	1.0	1.0	1.0	to	3' - <5'	0.70	0.77	0.79	0.19	-	-	-		70
10' x 10'	10	10	10	12	5' - 10'	0.79	0.81	0.87	0.18	_	-	-		64
	10	10	10	8	15'	1.09	1.19	1.23	0.15	-	-	-		52
	10 11	10	10	to	20' 25'	1.40 1.53	1.61 1.74	1.61 1.68	0.14	_	-	-		52 47
	12.5	11.5 12.5	10 10.5	12	30'	1.53	1.74	1.68	0.14	-	-	-		47

TAB	LE 15	- ST	ANDA	RD PRE	CAST BOX	CULVERT DESIGNS (3" COVER) - 11' SPANS								
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN	REINFORCEMENT AREAS								
(S) (R)	TOP	BOT.	I	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	A58	A59	(in.)
				4	0.33' - <2'	0.60	0.66	0.54	0.27	0.27	0.52	0.56		-
					2' - <3'	0.60	0.66	0.54	0.14	-	-	-		62
				to	3' - <5'	0.60	0.61	0.53	0.14	-	-	-		62
11' × 4'	11	11	11		5' - 10'	0.79	0.63	0.62	0.14	-	-	-		55
				12	15'	1.01	0.82	0.79	0.14	-	-	-		55
					20'	1.34	1.11	1.06	0.14	-	-	-		55
	12	12	11	8 to	25'	1.52	1.27	1.23	0.14	-	-	-		55
	13.5	13.5	11	12	30'	1.54	1.37	1.34	0.14	-	-	-		50
					0.33' - <2'	0.57	0.71	0.60	0.27	0.27	0.47	0.53		-
				4	2' - <3'	0.56	0.71	0.60	0.14	-	=	-		62
				to	3' - <5'	0.56	0.67	0.59	0.14	-	-	-		55
11' x 6'	11	11	11	12	5' - 10'	0.73	0.71	0.72	0.14	-	-	-		55
•				8	15'	0.92	0.92	0.91	0.14	-	-	-		50
	11	11	11		20'	1.21	1.25	1.21	0.14	-	-	-		50
	12	12	11	to	25' 30'	1.37	1.43	1.39	0.14	-	-	-		50
	13.5	13.5	11	12		1.39	1.53	1.50	0.14	- 27		-		50
				4	0.33' - <2'	0.55	0.76	0.66	0.27	0.27	0.46	0.55	5 5	-
				4	2' - <3'	0.55	0.76	0.66	0.14	-	-	-	Note	62
1.11 01	1.1	11	1,,	to	3' - <5'	0.54	0.72	0.65	0.14	-	-	-	<	62
11' x 8'	11	11	11	12	5' - 10'	0.73	0.79	0.82	0.14	-	_	-	General	55
	1.1	1 1	1 1	8	15'	0.93	1.03	1.03	0.14	-	-	-	епе	50
	11	11	11	to	20'	1.21	1.39	1.36	0.14	-	-	-	9	50 50
	12 13.5	12.5 13.5	11	12	25' 30'	1.34 1.41	1.56 1.66	1.50 1.65	0.14	-	-	-	See	50
	13.5	13.5	11	12	0.33' - <2'	-				0.27	0.40		,	-
				4	0.33 - <2 2' - <3'	0.60	0.81 0.81	0.71	0.27	0.27	0.48	0.60		75
				to	2 - <3 3' - <5'	0.61	0.77	0.71	0.13	_	_	<del>-</del>		69
11' × 10'	11	11	11	12	5' - 10'	0.80	0.77	0.70	0.14	_	_	_		62
	1 1	11	''	12	15'	1.01	1.13	1.15	0.14	_				55
	11	11	11	8	20'	1.30	1.52	1.52	0.14	_	_			50
	12	12.5	11	to	25'	1.42	1.70	1.65	0.14	_	_	_		50
	13.5	14	11	12	30'	1.53	1.77	1.74	0.14	_	_	_		50
	10.0	17	11		0.33' - <2'	0.64	0.83	0.74	0.27	0.27	0.51	0.64		-
				4	2' - <3'	0.64	0.83	0.74	0.21	-	-	-		86
				to	3' - <5'	0.67	0.79	0.75	0.21	_		_		75
11' × 11'	11	11	11	12	5' - 10'	0.88	0.93	0.99	0.19	_	_	_		69
		**	''		15'	1.09	1.19	1.23	0.16	_	_	_		55
	11	11	11	8	20'	1.40	1.59	1.60	0.15	_	_	_		55
	12	12.5	11	to	25'	1.54	1.77	1.73	0.15	_	_	-		50
	13.5	14	11.5	12	30'	1.57	1.77	1.76	0.14	_	_	_		50
	10.0		111.5	_		1.0.		1	,	l	<u> </u>			

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
   See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

≥ DESCRIPTION:



T.	ABLE	16 - 5	STANI	DARD P	RECAST BO	X CU	LVERT	DES	IGNS	(3" (	COVER	R) - 12	2' SP/	4 <i>NS</i>
SPAN x RISE	SLAE	/ WAL	L THIC	KNESS	DESIGN			R	EINFOF	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	ВОТ.	_	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.59	0.64	0.51	0.29	0.29	0.52	0.55		-
				4	2' - <3'	0.60	0.64	0.51	0.15	-	-	-		73
	12	12	12	to	3' - <5'	0.60	0.61	0.51	0.15	-	-	-		66
12' x 4'	12	12	12	10	5' - 10'	0.81	0.61	0.61	0.15	-	-	-		66
12 / 7				12	15'	1.04	0.80	0.77	0.15	-	-	-		59
				12	20'	1.37	1.08	1.03	0.15	-	-	-		59
	13	13	12	8 to	25'	1.58	1.26	1.21	0.15	-	-	-		59
	14.5	14.5	12	12	30'	1.63	1.38	1.34	0.15	-	-	-		53
				4	0.33' - <2'	0.56	0.70	0.57	029	0.29	0.47	0.52		-
				4	2' - <3'	0.56	0.70	0.57	0.15	-	-	-		66
12' x 6'	12	12	12	t o	3' - <5'	0.56	0.67	0.57	0.15	-	-	-		59
		12	12	to	5' - 10'	0.74	0.69	0.70	0.15	-	-	-		59
				12	15'	0.94	0.90	0.88	0.15	-	-	-		53
				12	20'	1.23	1.22	1.17	0.15	-	-	-		53
	13	13	12	8 to	25'	1.40	1.42	1.37	0.15	_	-	_		53
	14.5	15	12	12	30'	1.44	1.54	1.48	0.15	-	-	-		53
	12 12				0.33' - <2'	0.55	0.75	0.63	0.29	0.29	0.45	0.53		-
				4	2' - <3'	0.55	0.75	0.63	0.15	_	-	-	Note	66
		12	12	to	3' - <5'	0.55	0.73	0.63	0.15	-	-	-		59
12' x 8'			12	12	5' - 10'	0.73	0.77	0.79	0.15	-	-	-	General	59
12 / 0					15'	0.93	1.00	0.99	0.15	-	-	-	neı	53
	12	12	12	8	20'	1.21	1.35	1.31	0.15	-	-	-		53
	13	13.5	12	to	25'	1.35	1.55	1.48	0.15	_	-	-	See	53
	14.5	15	12	12	30'	1.40	1.67	1.62	0.15	-	-	-	S	53
					0.33' - <2'	0.57	0.80	0.68	0.29	0.29	0.46	0.57		-
				4	2' - <3'	0.57	0.80	0.68	0.15	-	-	-		73
	12	12	12	to 12	3' - <5'	0.59	0.77	0.68	0.15	-	-	-	]	66
12' x 10'	12	12	12		5' - 10'	0.78	0.85	0.89	0.15	-	-	-		59
12 × 10					15'	0.98	1.10	1.11	0.15	-	-	-		53
	12	12	12	8	20'	1.26	1.47	1.45	0.15	-	-	-		53
	13	13.5	12	to	25'	1.39	1.68	1.63	0.15	-	-	-		53
	14.5	15	12	12	30'	1.48	1.79	1.76	0.15	-	-	-		53
					0.33' - <2'	0.65	0.84	0.73	0.29	0.29	0.50	0.65		-
				4	2' - <3'	0.65	0.84	0.73	0.23	-	-	-		93
	12	12	12	to	3' - <5'	0.68	0.81	0.75	0.22	-	-	-		80
12' x 12'	12	12	12	12	5' - 10'	0.90	0.94	1.01	0.21	-	-	-		73
12 12					15'	1.12	1.20	1.24	0.18	-	-	-		59
	12	12	12	8	20'	1.42	1.60	1.61	0.16	-	-	-		59
	13	13.5	12	to	25'	1.57	1.81	1.78	0.16	-	-	-		53
				1					1		1	1	i	1

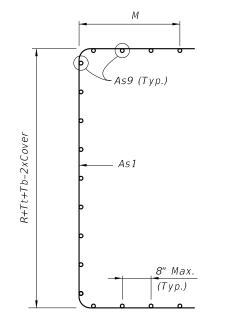
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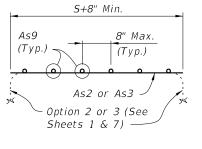
14.5 15 12.5

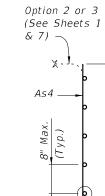
- 1. See Sheet 2 of 14 for General Notes.
- 2. See Sheet 7 of 14 for Reinforcing Details and dimension locations.

1.63 1.86 1.85 0.15

## WELDED WIRE REINFORCEMENT BENDING DIAGRAM

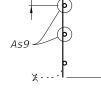






WWR PIECE NO. 2 (2 Reqd. per segment)

S+2(Tw+10"-Cover-M) As9 8" Max. (Тур.) (Typ.)As4 (3 Wires Min.)

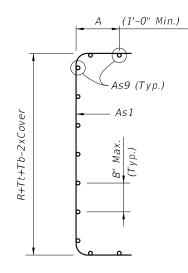


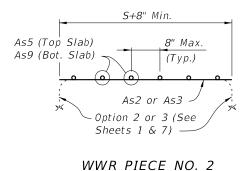
WWR PIECE NO. 1 (2 Regd. per segment)

WWR PIECE NO. 4 (Tongue Reinforcement) (4 Regd. per segment)

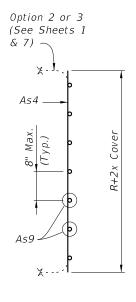
WWR PIECE NO. 3 (2 Regd. per segment)

## TYPE 2 BOX SECTION (DESIGN EARTH COVER 2' OR GREATER)





(2 Reqd. per segment) S+2(Tw+10"-Cover-A) 8" Max. (Typ.)As7 (Top Slab) As8 (Bot. Slab)



WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (2 Regd. per segment)

WWR PIECE NO. 3 (2 Reqd. per segment)

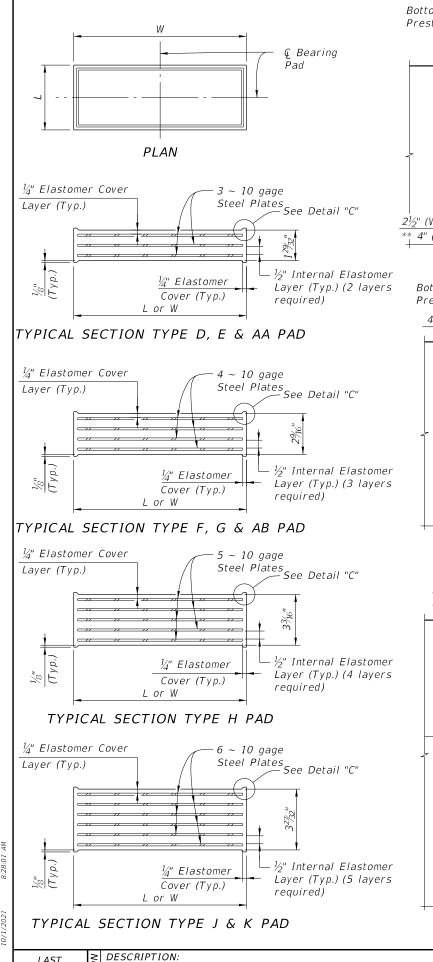
# TYPE 1 BOX SECTION (DESIGN EARTH COVER LESS THAN 2')

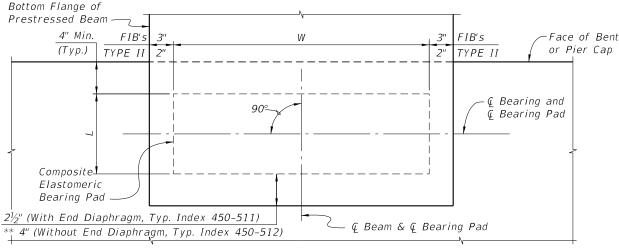
#### REINFORCEMENT NOTES:

- 1. Reinforcement bending dimensions are out-to-out.
- 2. See General Notes 4, 5 and 6 on Sheet 2.
- 3. See Tables 1 thru 16 for dimensions M, R, S, Tb, Tt and Tw.
- 4. Dimension "A" is determined by the Manufacturer in accordance with the requirements of Detail "B" on Sheets 1 and 7.

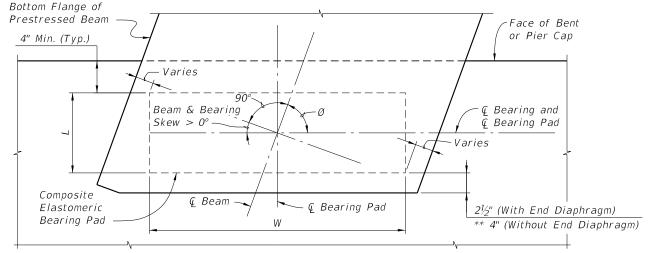
DESCRIPTION:

53

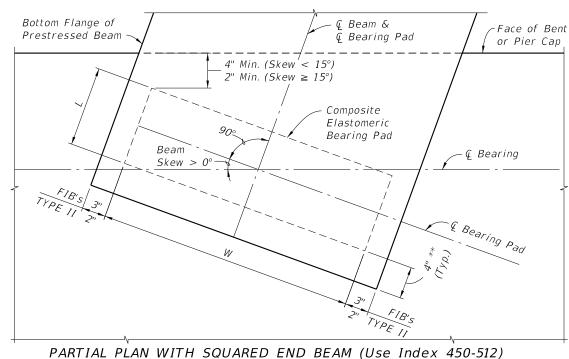




PARTIAL PLAN (Beam & Bearing Skew =  $0^{\circ}$ )



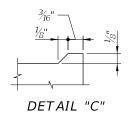
PARTIAL PLAN (Beam & Bearing Skew > 0°) (Use Index 450-511)



(Beam Skew >  $0^{\circ}$ ; Bearing Skew =  $0^{\circ}$ )

\*BEVELED BEARING PAD BEARING PLATE BEAMTYPEDIMENSIONS DIMENSIONS PAD TYPE D (See Note 1 D 2'-8" 1'-0" 3'-0" (G=110psi) Ε 10" 2'-8" 3'-0" 1'-0" (G=110psi)3'-0" 10" 2'-8" 1'-0" (G=110psi)G 10" 2'-8" 1'-0" 3'-0" (G=150psi)Н 3'-0" 10" 2'-8" 1'-0" (G=150psi) 10" 2'-8" 1'-0" 3'-0" (G=150psi)Κ 1'-0" 1'-11/2" 3'-0" 2'-8" (G=150psi) AA10" 1'-0" 1'-4" (G=110psi) AB1'-2" 1'-0" 1'-4" (G=150psi)

- \* Work this sheet with the appropriate type Bearing Plate Detail (See Bearing Plate Data Table) and BEARING PAD DATA TABLE in the Structures Plans. See TABLE OF BEAM VARIABLES and BEARING PLATE DATA TABLE in the Structures Plans for locations where beveled bearing plates are required.
- \*\* Offset to End of Beam is reduced to 2" for Type K Pad.



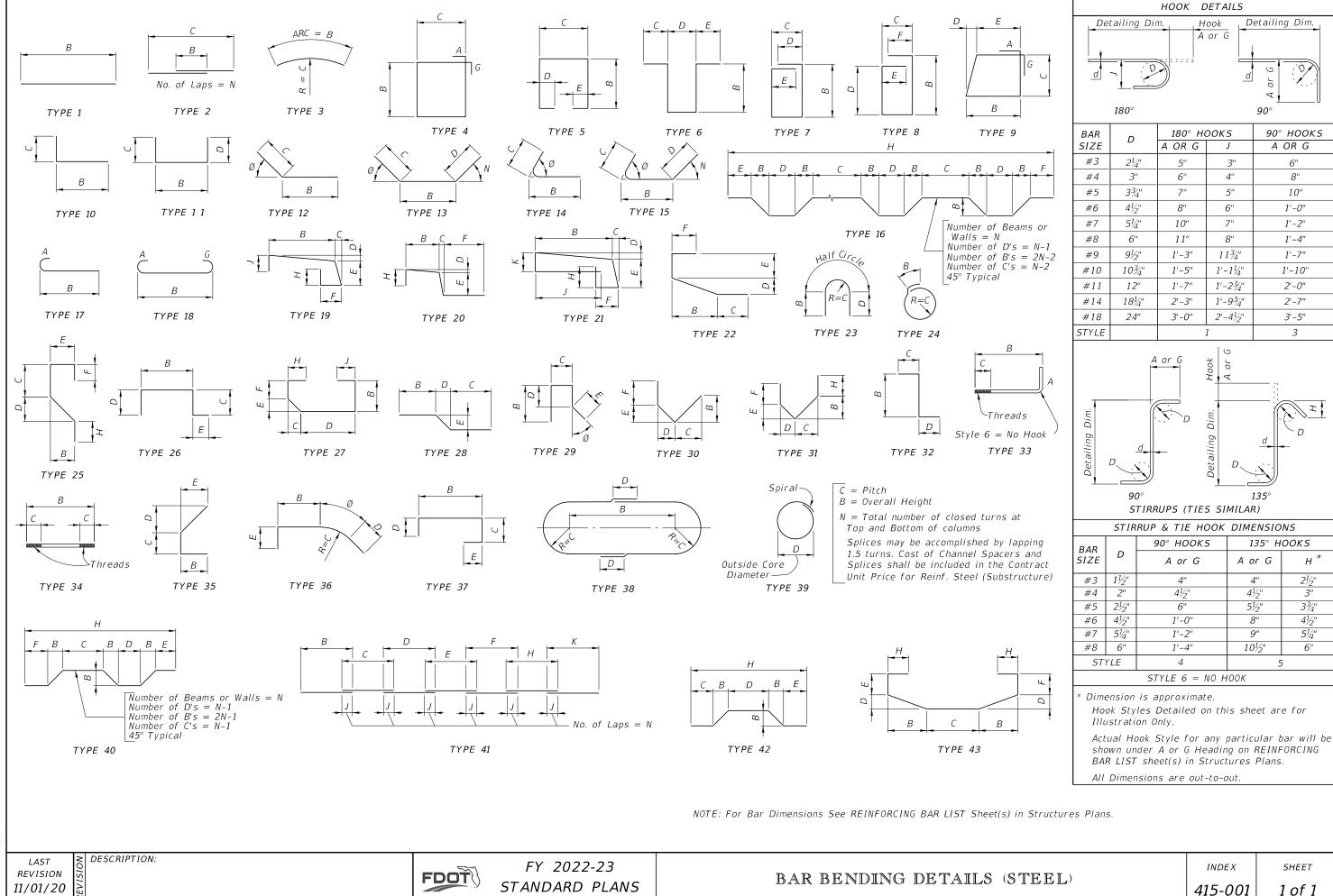
### **BEARING PAD NOTES:**

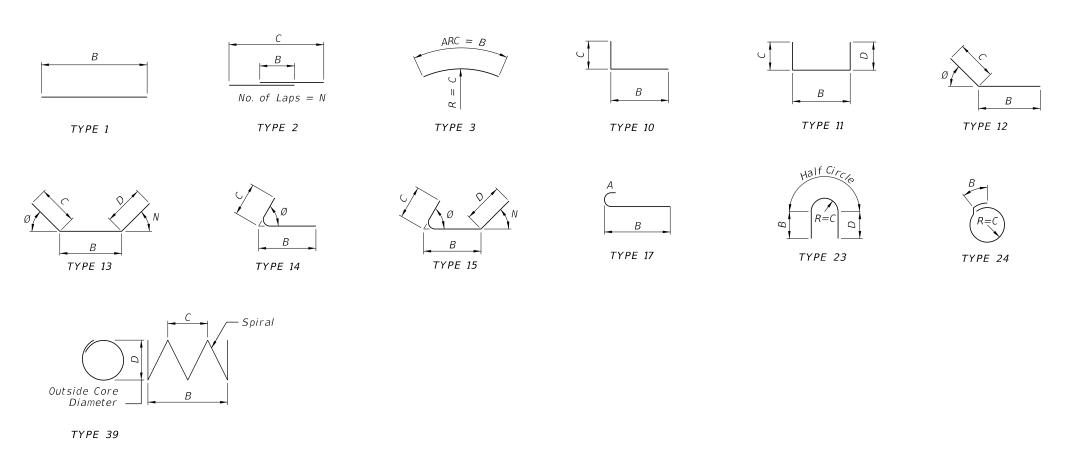
- 1. Neoprene in Type D, E, F & AA bearing pads shall have a shear modulus (G) of 110 psi. Neoprene in Type G, H, J, K & AB bearing pads shall have a shear modulus (G) of 150 psi.
- 2. Steel Plates in bearing pads shall conform to ASTM A1011 Grade 36, Type 1.
- 3. See Bearing Pad Data Table in Structures Plans for quantities of Type D, E, F, G, H, J, K, AA and/or AB Bearing Pads

400-510 1 of 1

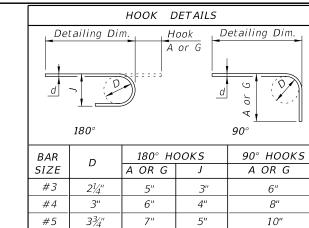
REVISION

07/01/15





🗆 SINGLE BAR BENDING DETAILS 💳



NOT	rec
1401	LJ

8"

10"

11"

#### GENERAL

#6

#7

#8

All dimensions are out-to-out.

For Bar dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

6"

7"

8"

6"

8"

10"

1'-0"

1'-2"

1'-4"

### SPIRALS (TYPE 39 BARS)

C = Pitch

B = Overall Height

4½"

5¼"

6"

N = Total number of closed turns at

Top and Bottom of columns

Spirals = 1.5 turns

Include spiral splice in Contract Unit Price for FRP Reinforcing.

### H00K5

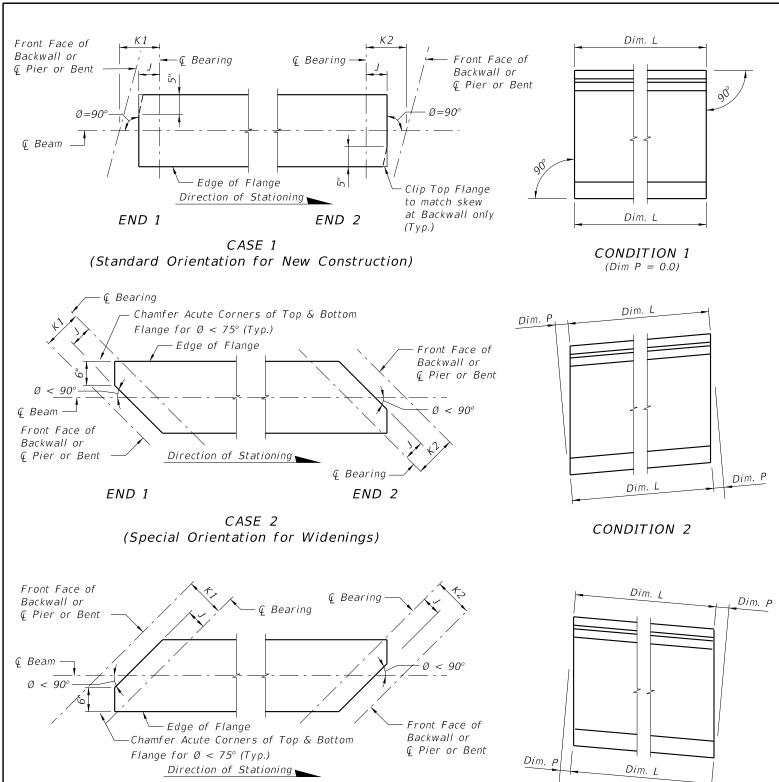
All Dimensions are approximate.

Hook Styles Detailed on this sheet are for Illustration Only. Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.

REVISION 11/01/20

DESCRIPTION:





END 2

CASE 3

(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

BEAM NOTES

- 1. Work this Index with the Florida-I Beam Standard Details (Index 450-036 thru 450-096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum.
- 4. Stress Strands N to 10 kips each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
  - A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands.
  - B. For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
  - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
  - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
  - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.
- D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement.
- 10. Contractor Options:
  - A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
  - B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
  - A. The superstructure environmental classification is slightly or moderately aggressive
  - B. Clear cover to adjacent steel reinforcing is 1"or greater
  - C. Hole inside diameter is 2" maximum
  - D. Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

8:28:13

10/1/2021

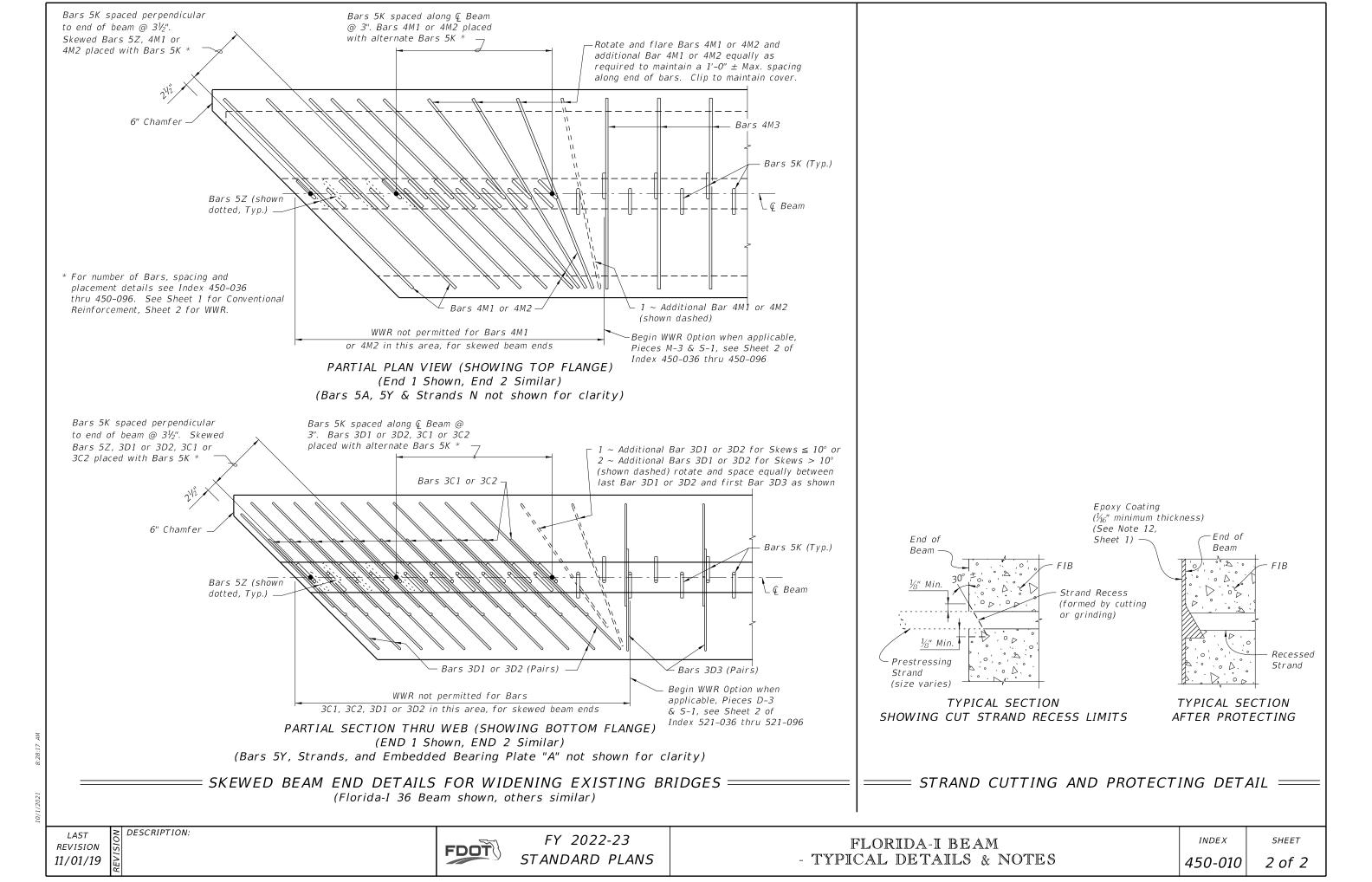
LAST ODESCRIPTION:
REVISION 151
11/01/21

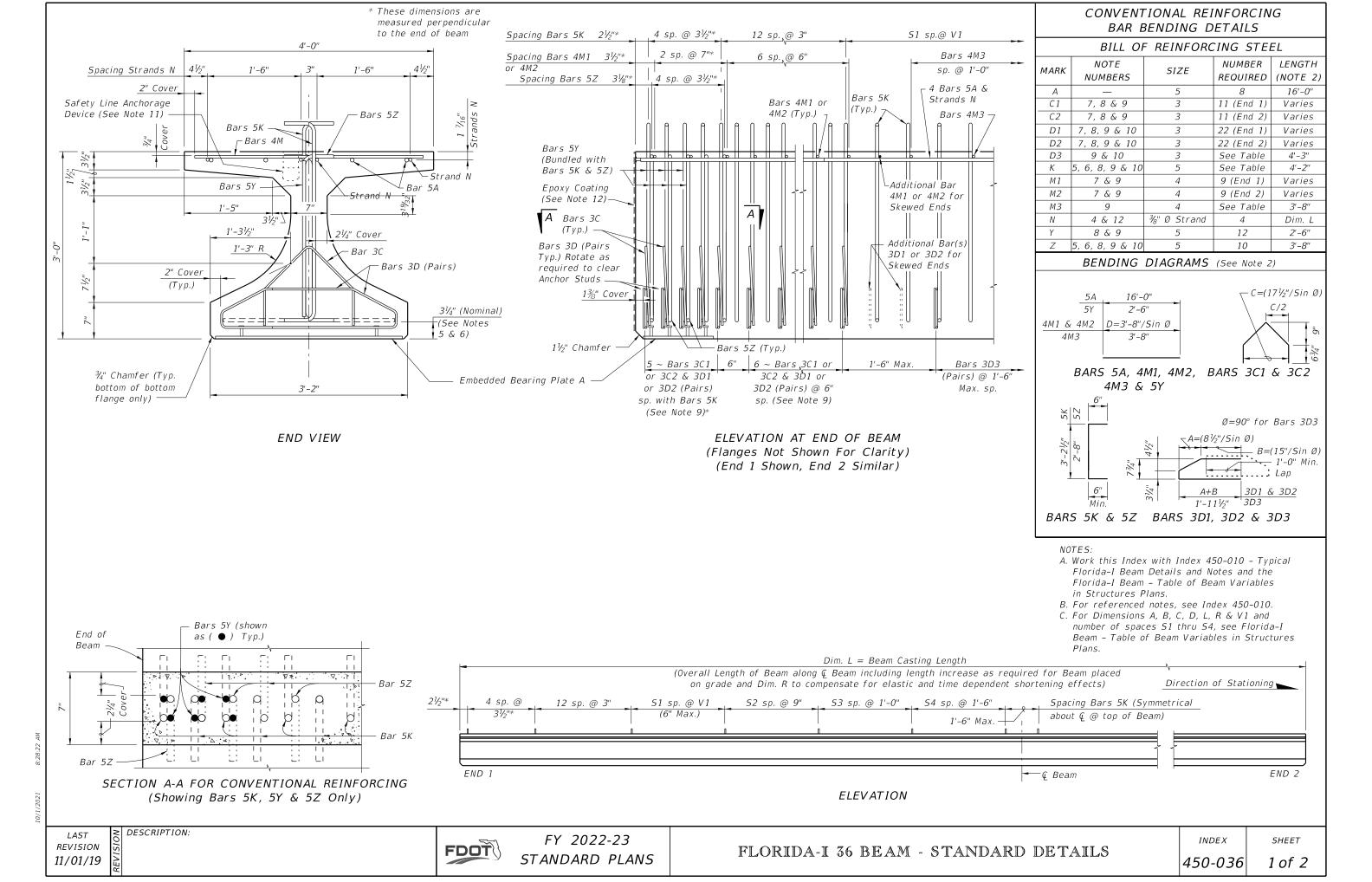
END 1



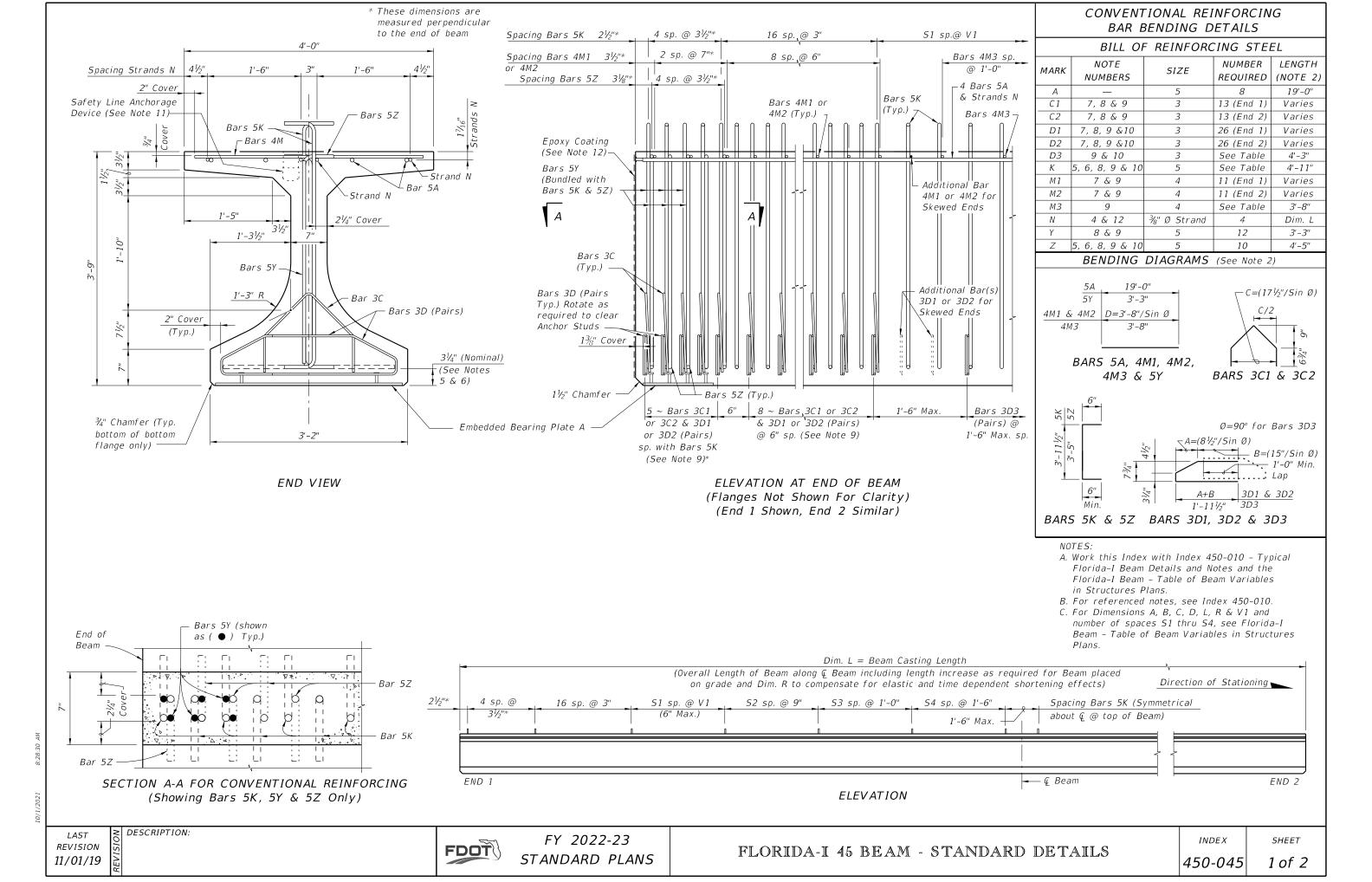
CONDITION 3

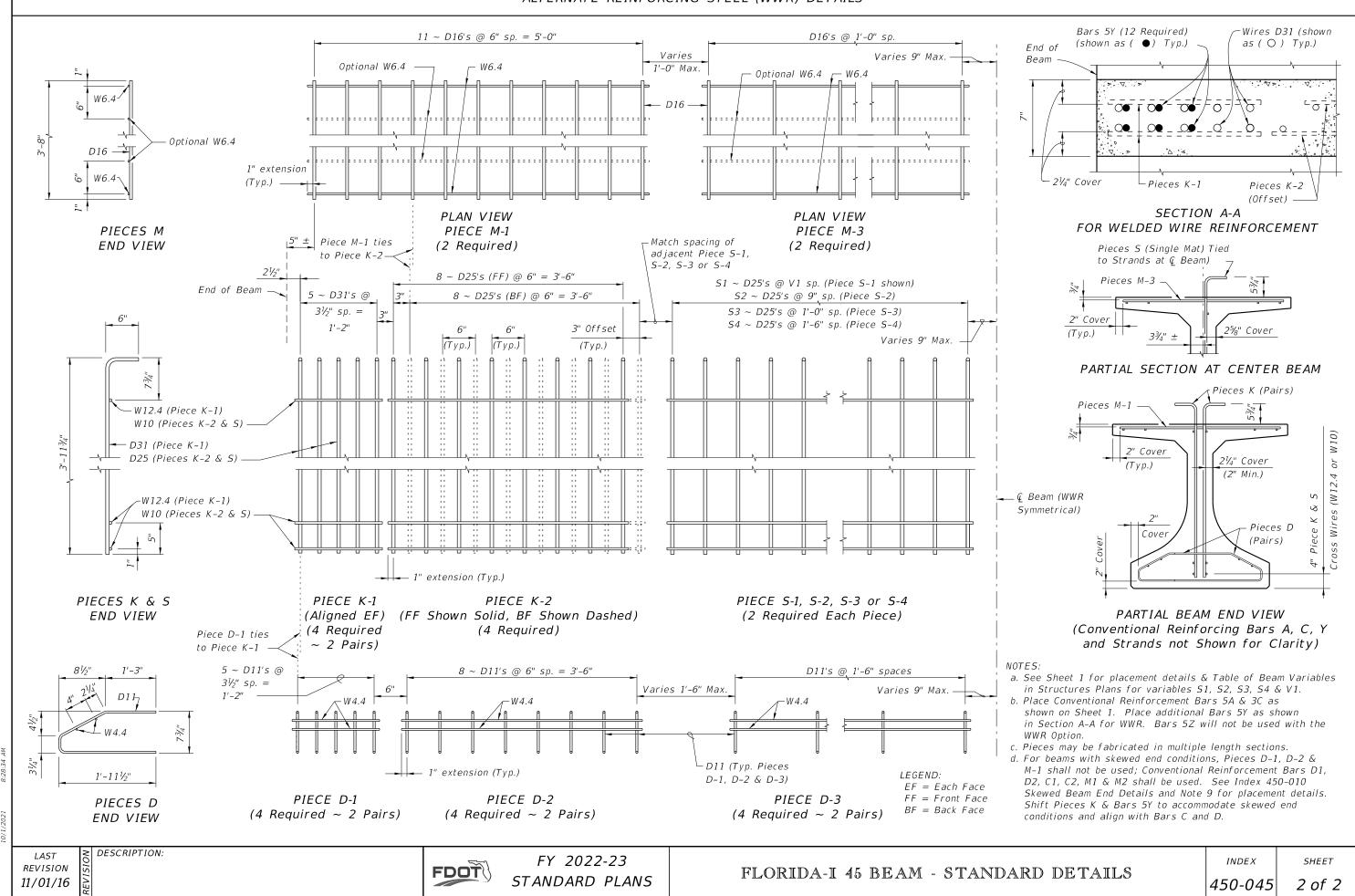
SCHEMATIC END ELEVATIONS OF BEAMS
(Showing Vertical Bevel of Beam End)

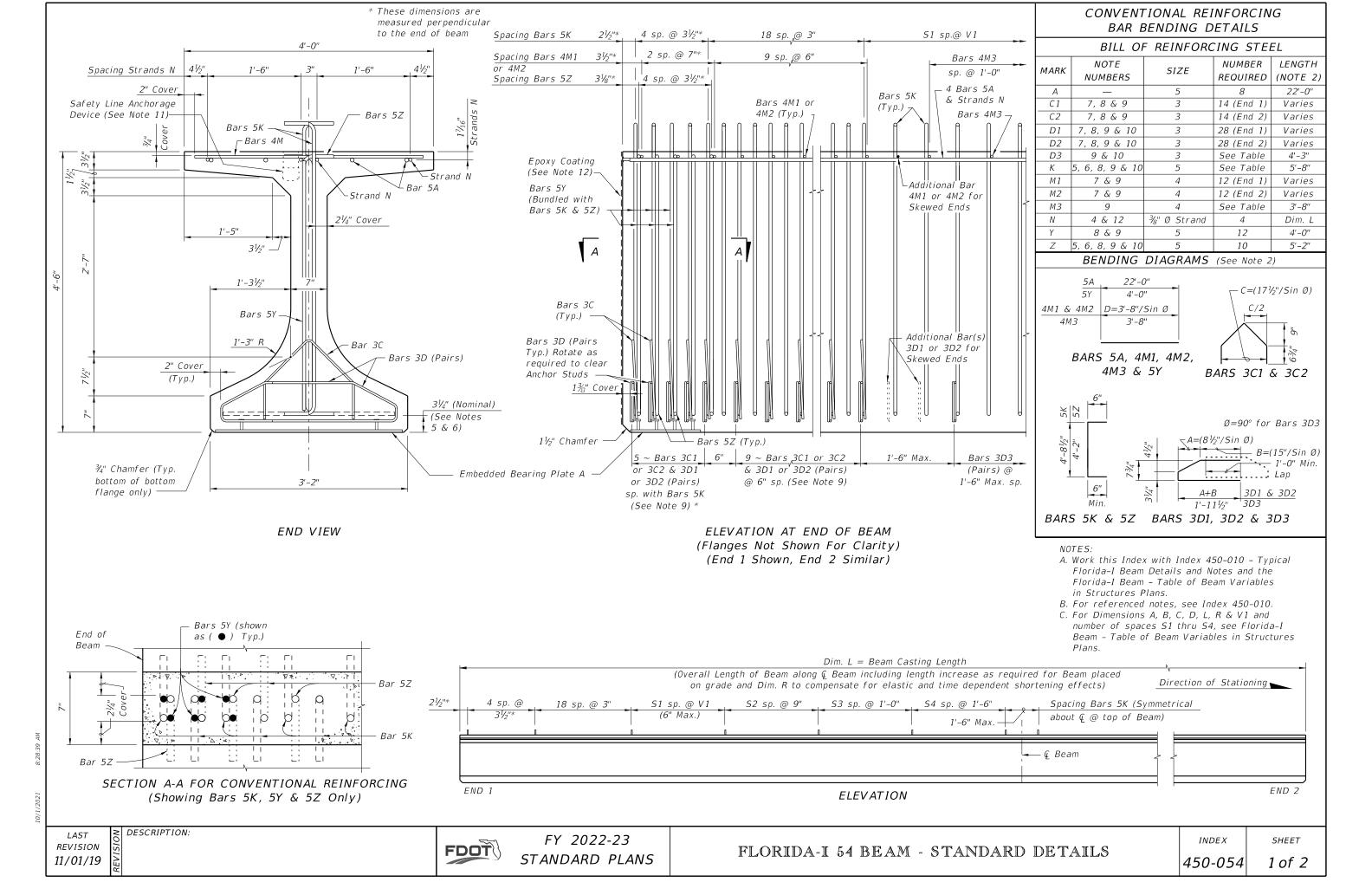


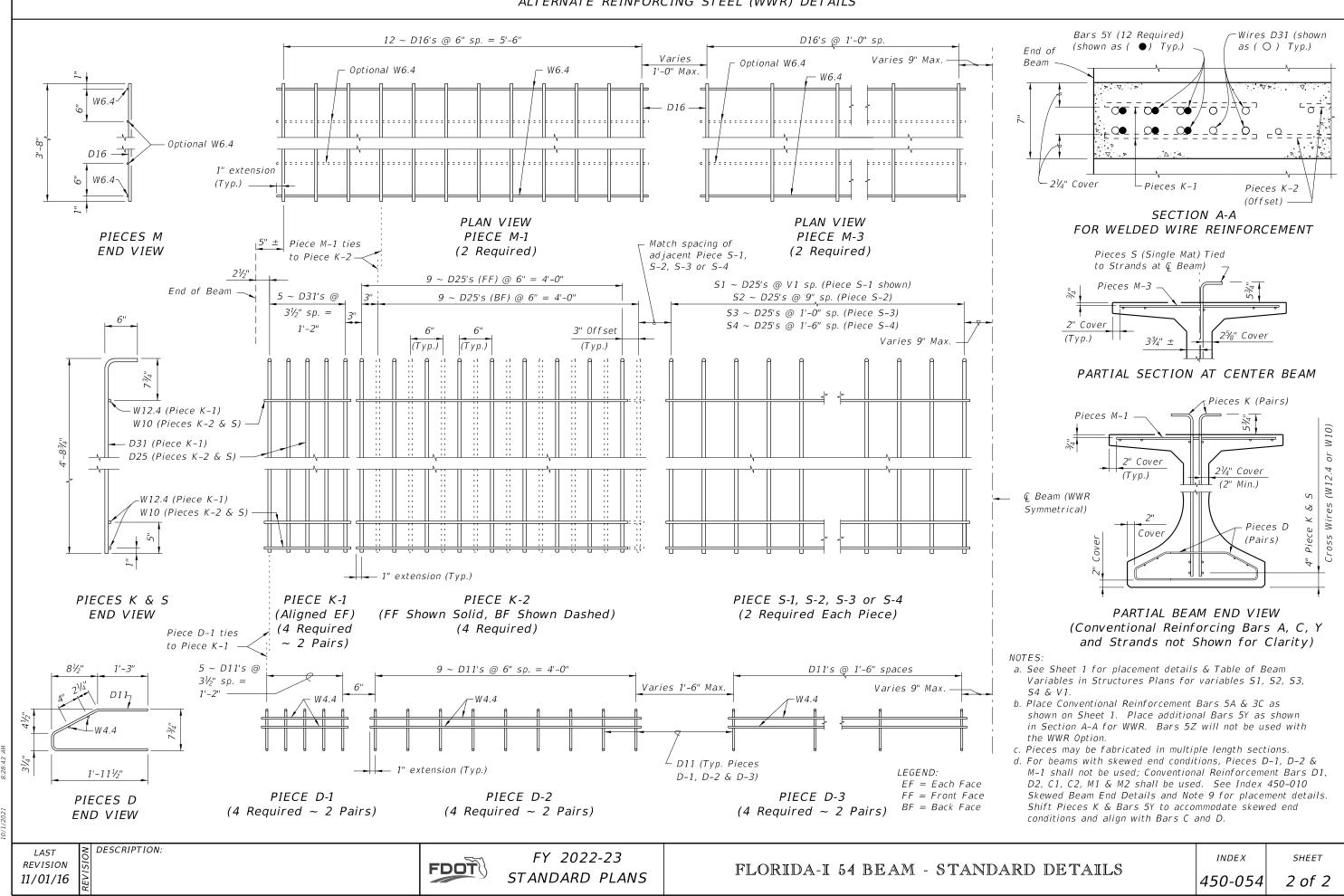


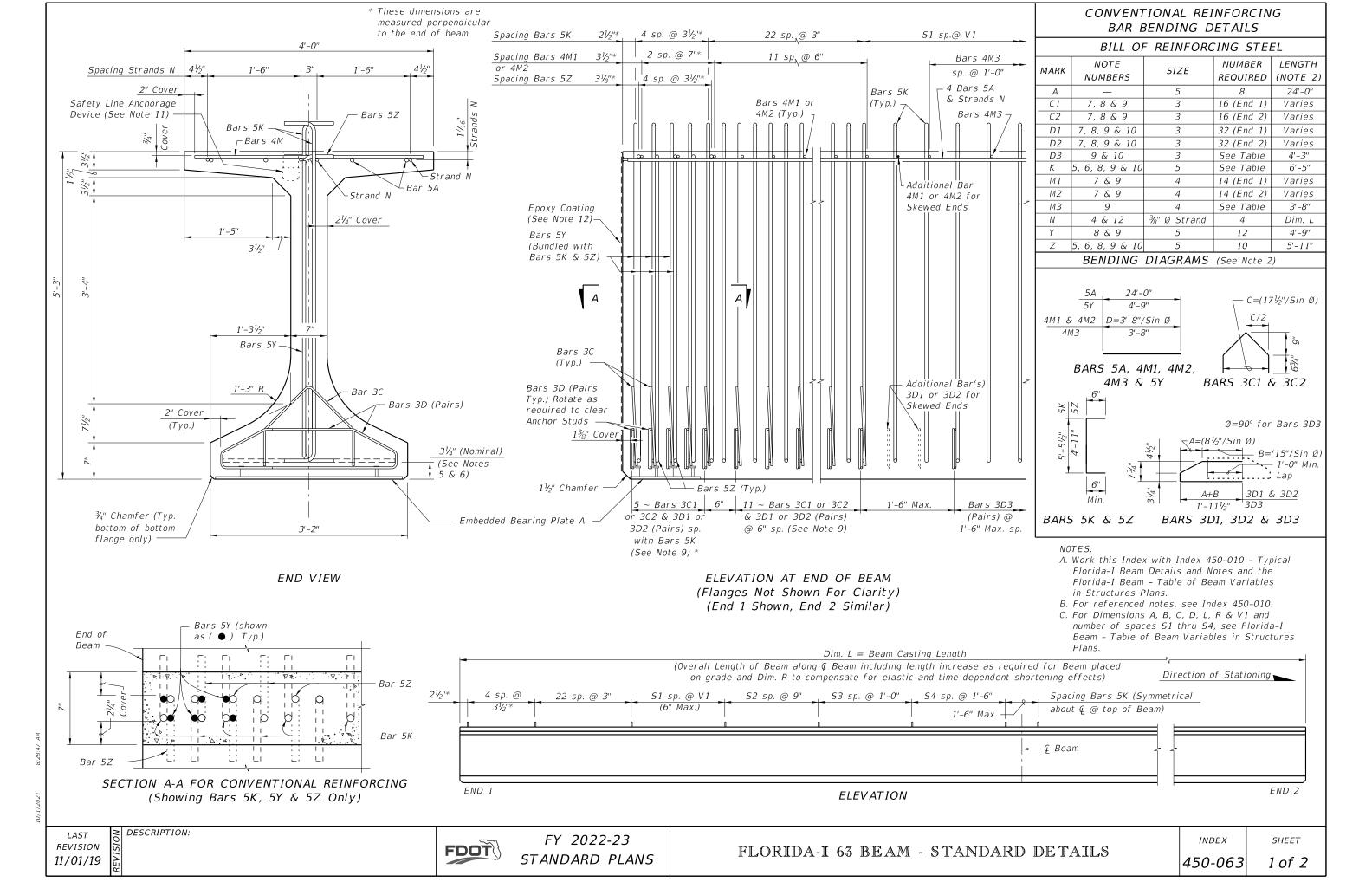
#### ALTERNATE REINFORCING STEEL (WWR) DETAILS 9 ~ D16's @ 6" sp. = 4'-0" D16's @ 1'-0" sp. Bars 5Y (12 Required) -Wires D31 (shown (shown as ( ●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max Optional W6.4 -W6.4 1'-0" Max. Beam — Optional W6.4 - W6.4 W6.4-D16-Optional W6.4 D16 ----1" extension W6.4 (Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) PLAN VIEW PLAN VIEW SECTION A-A PIECES M PIECE M-1 PIECE M-3 – Match spacing FOR WELDED WIRE REINFORCEMENT 5" ± Piece M-1 ties END VIEW of adjacent (2 Required) (2 Required) to Piece K-2 Piece S-1, S-2, Pieces S (Single Mat) Tied 6 ~ D25's (FF) S-3 or S-4 21/5" to Strands at © Beam) S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam — Pieces M-3 6 ~ D25's (BF) S2 ~ D25's @ 9" sp. (Piece S-2) @ 6'' = 2'-6'' $3\frac{1}{2}$ " sp. = S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 2" Cover Varies 9" Max. 25/8" Cover (Typ.)(Typ.) (Typ.)(Typ.)3¾" ± PARTIAL SECTION AT CENTER BEAM W12.4 (Piece K-1) Pieces K (Pairs) W10 (Pieces K-2 & S) Pieces M-1 W10) or D31 (Piece K-1 (W12.4 2" Cover D25 (Pieces K-2 & S) -21/4" Cover (Typ.)(2" Min.) Ø Wires ( .\_\_Ç Beam (WWR -W12.4 (Piece K-1) Symmetrical) Pieces D W10 (Pieces K-2 & S) Piece Cover (Pairs) 1" extension (Typ.) PARTIAL BEAM END VIEW PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 (Conventional Reinforcing Bars A, C, Y END VIEW (Aligned EF) (FF Shown Solid, (2 Required Each Piece) and Strands not Shown for Clarity) (4 Required ~ BF Shown Dashed) Piece D-1 ties to Piece K-1 2 Pairs) (4 Required) NOTES: a. See Sheet 1 for placement details & Table of Beam 5 ~ D11's @ $6 \sim D11's @ 6'' sp. = 2'-6''$ D11's @ 1'-6" spaces 1'-3" Variables in Structures Plans for variables S1, S2, S3, $3\frac{1}{2}$ " sp. = Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & M-1 shall not be used; Conventional Reinforcement Bars D1, D11 (Typ. Pieces - 1" extension (Typ.) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index 450-010 D-1, D-2 & D-3) EF = Each FaceSkewed Beam End Details and Note 9 for placement details. PIECE D-1 PIECE D-2 PIECE D-3 FF = Front Face Shift Pieces K & Bars 5Y to accommodate skewed end PIECES D BF = Back Faceconditions and align with Bars C and D. (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) **END VIEW** DESCRIPTION: FY 2022-23 INDEX SHEET REVISION FDOT FLORIDA-I 36 BEAM - STANDARD DETAILS STANDARD PLANS 11/01/16 450-036 2 of 2

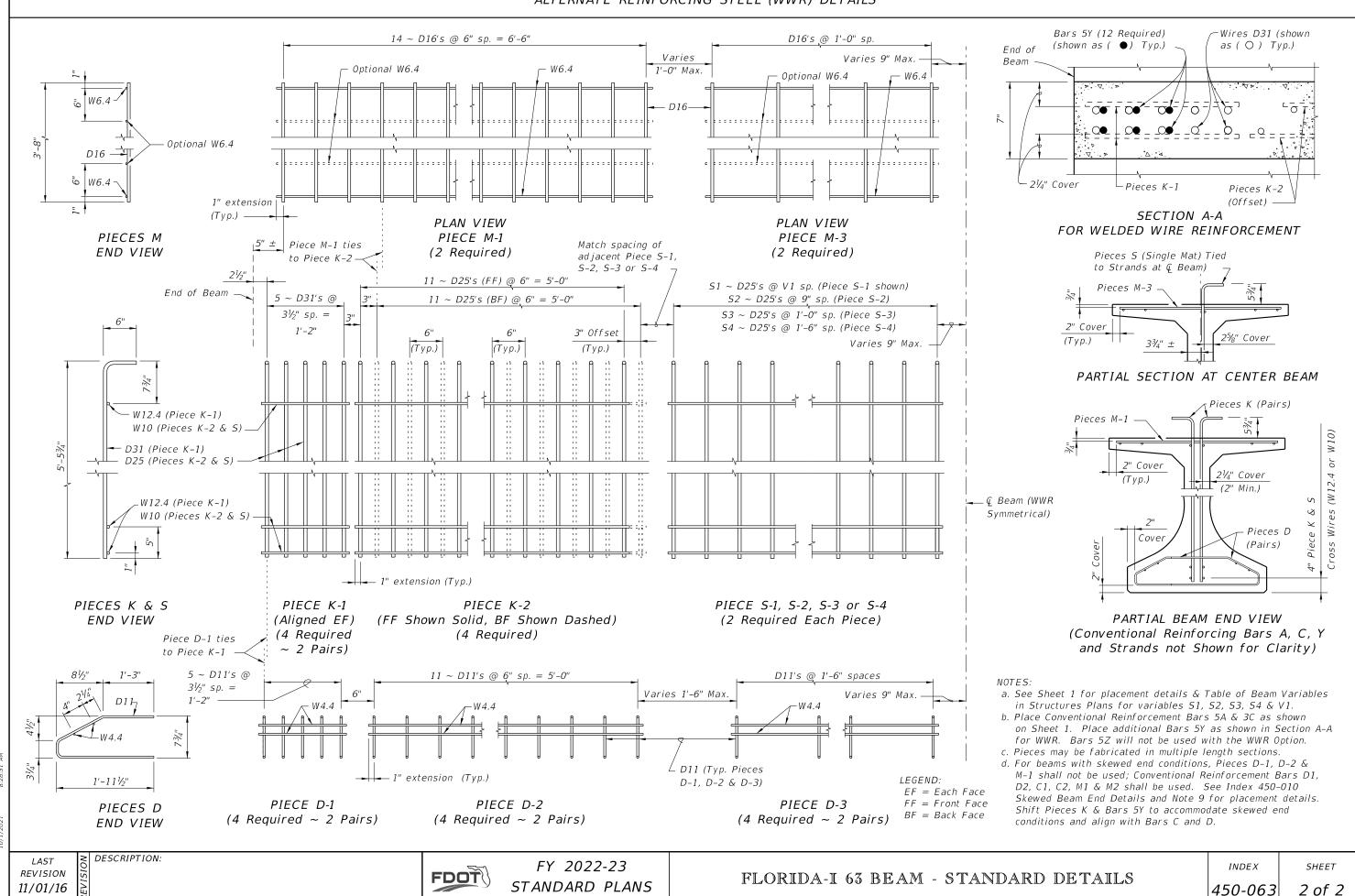


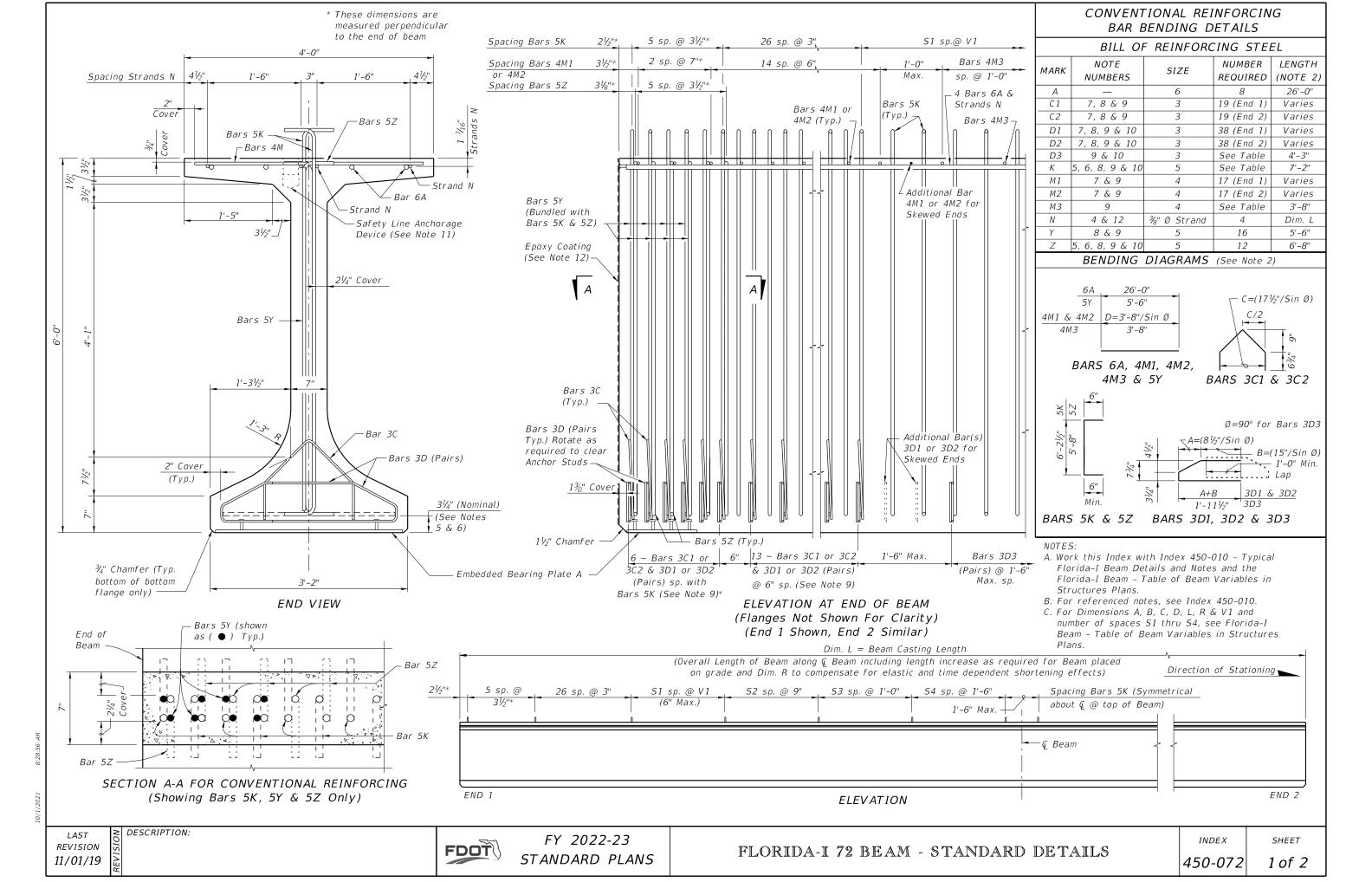


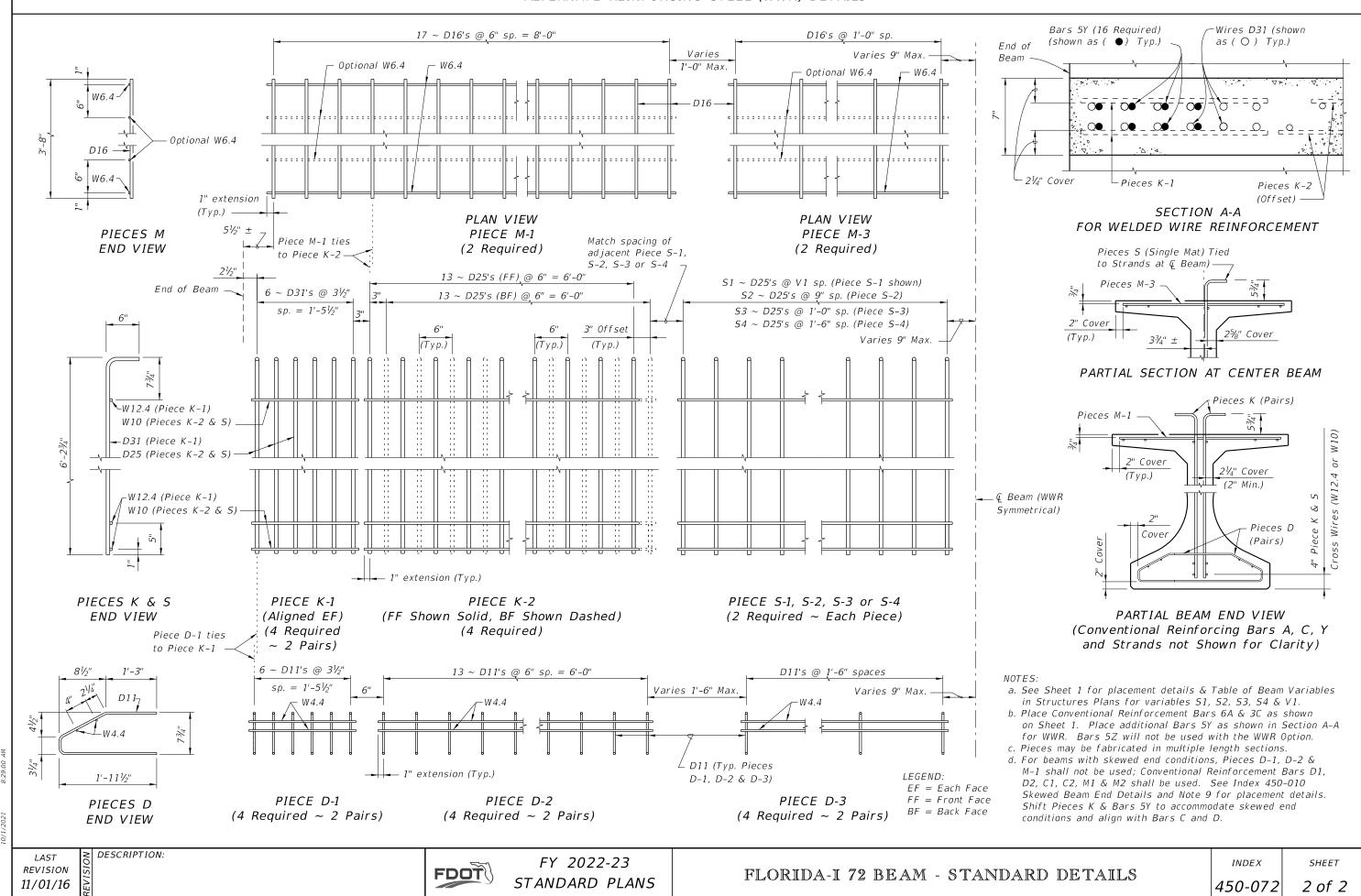


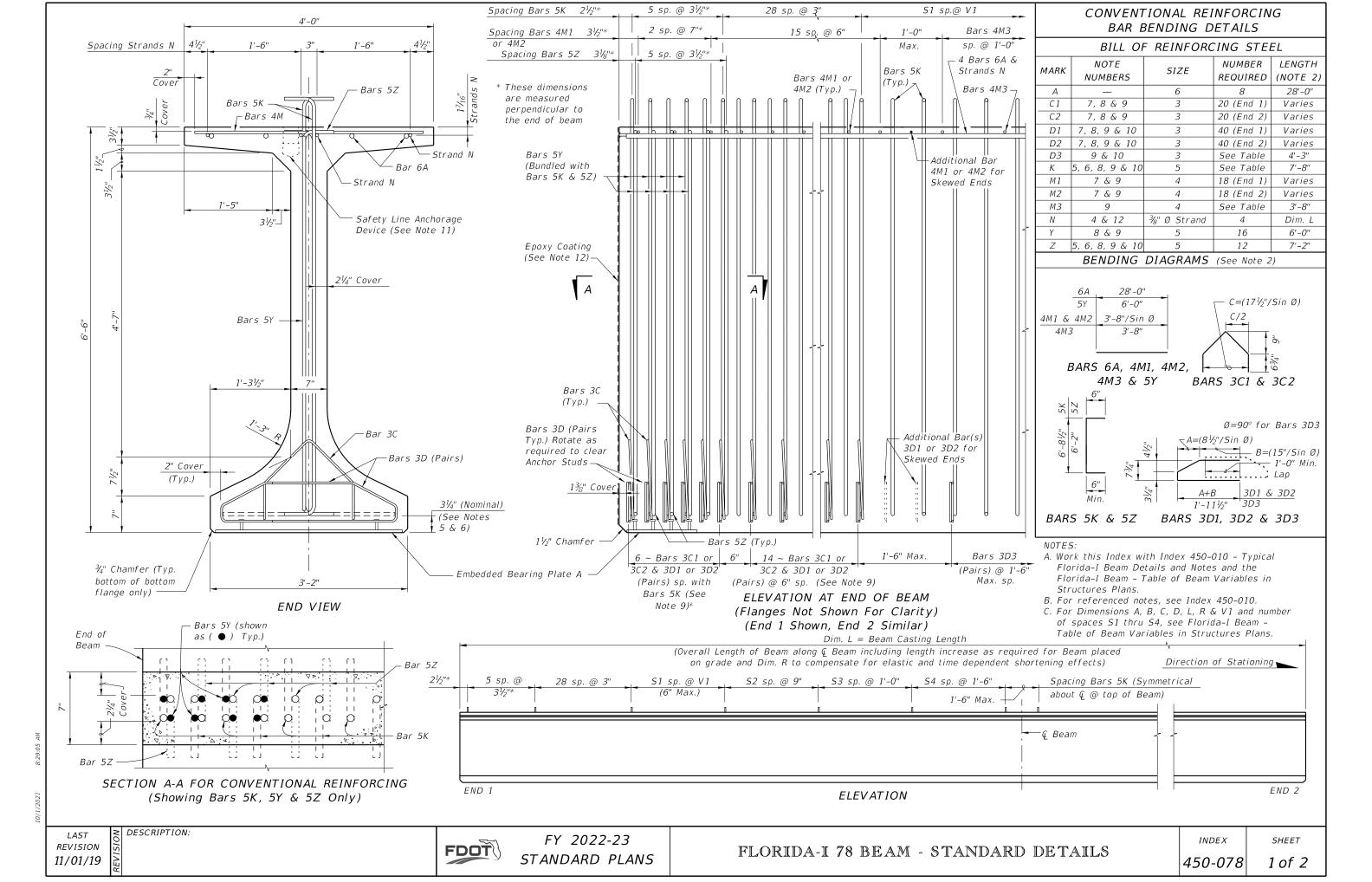


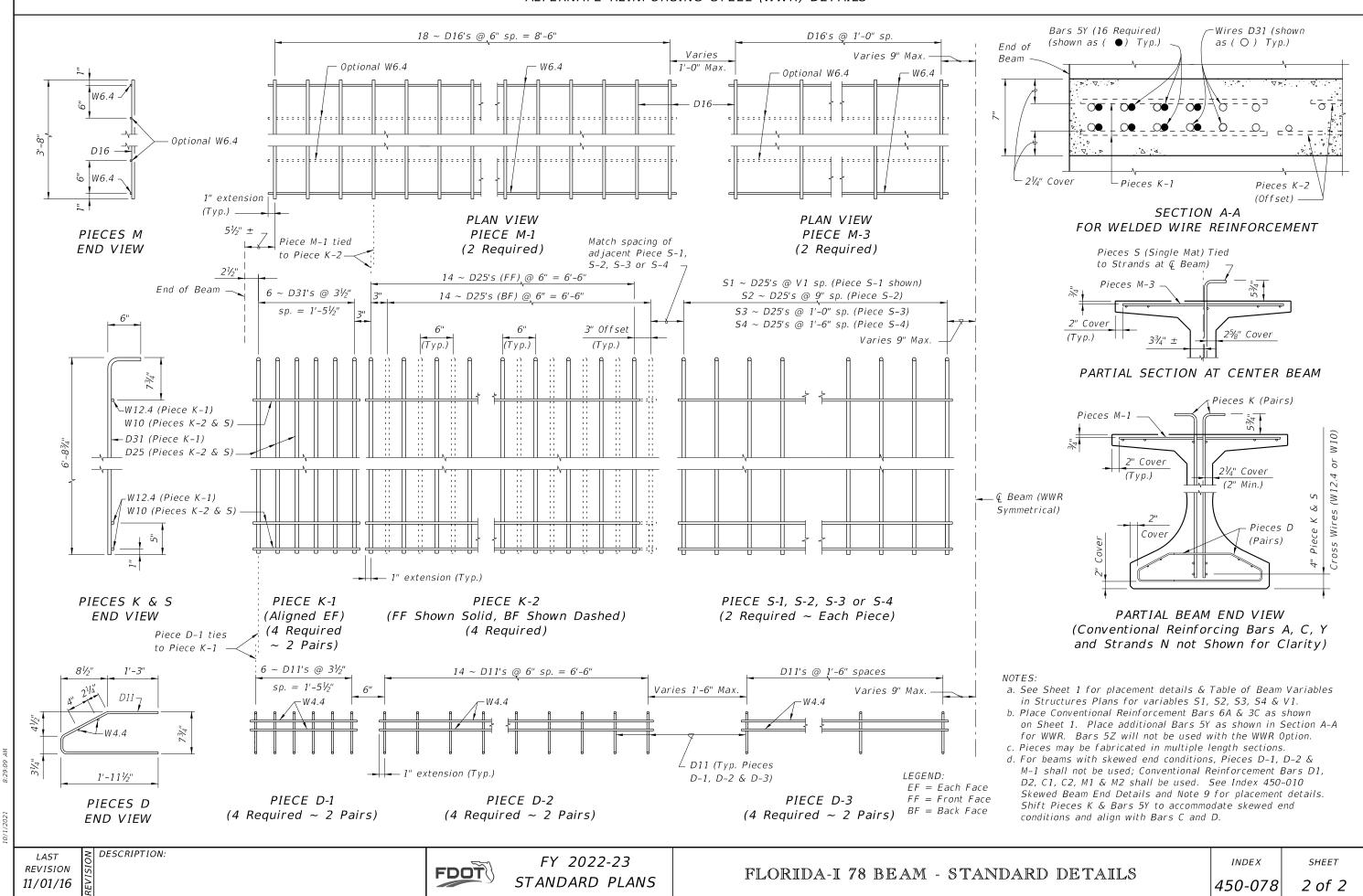


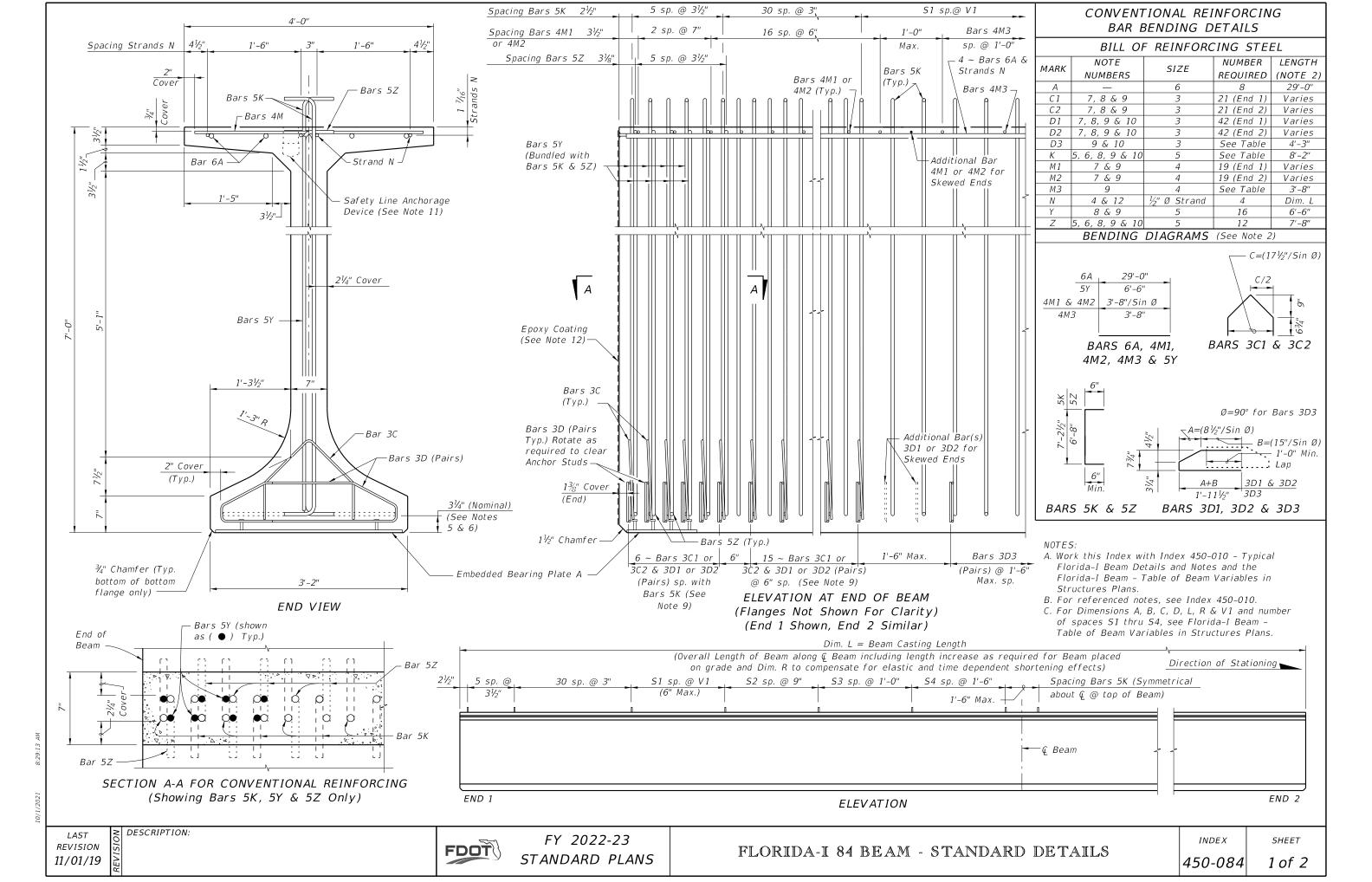


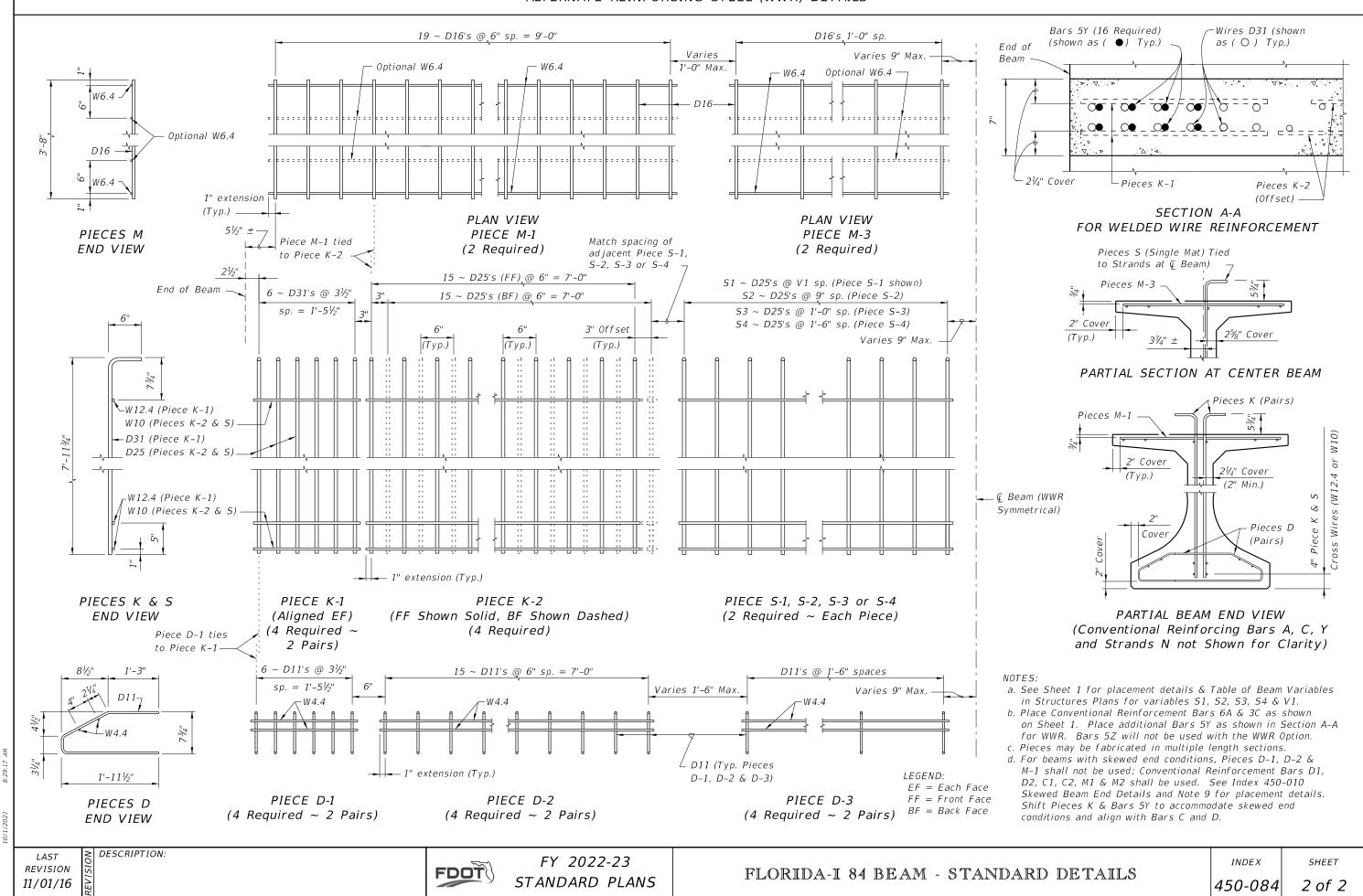


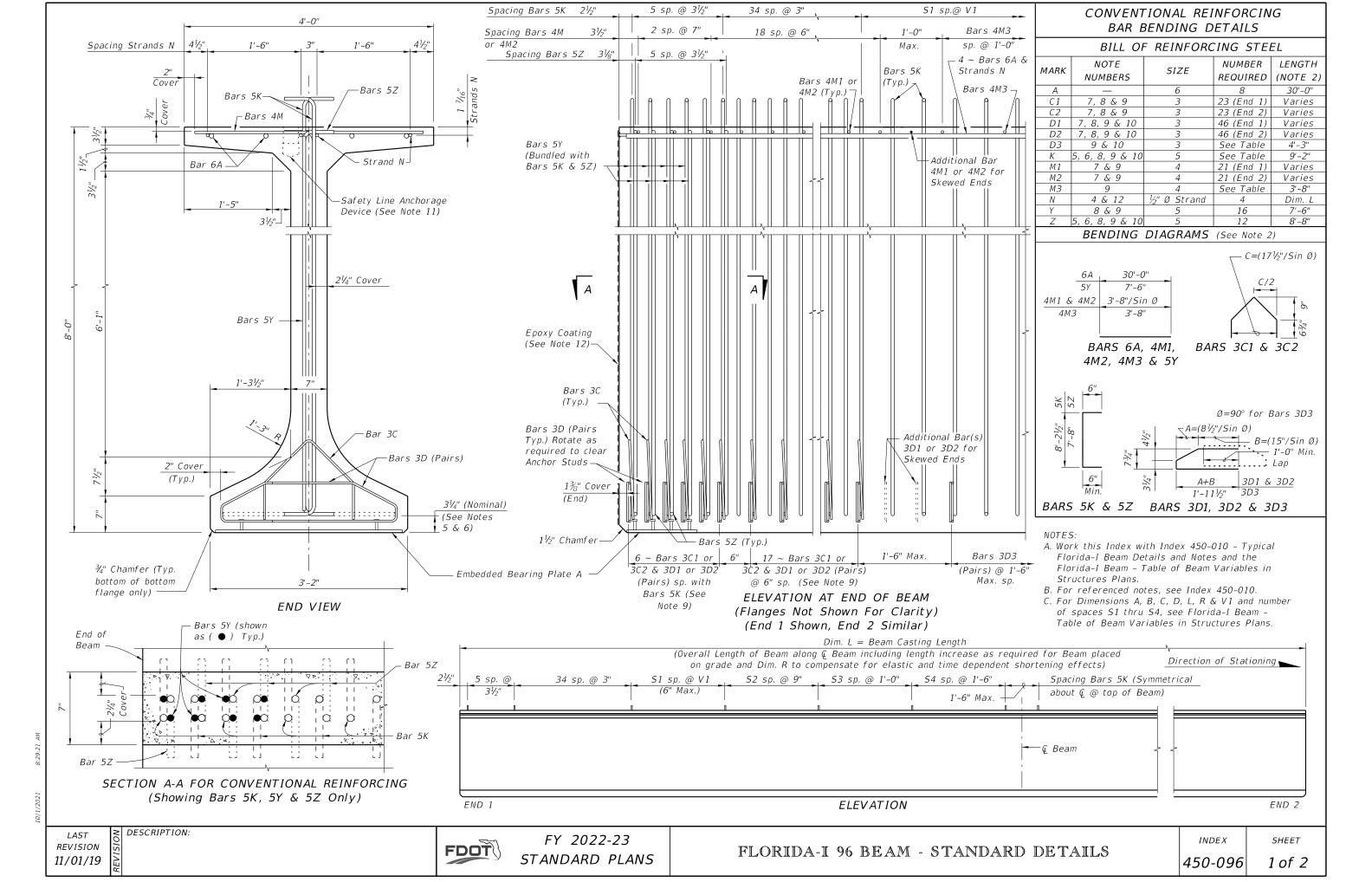


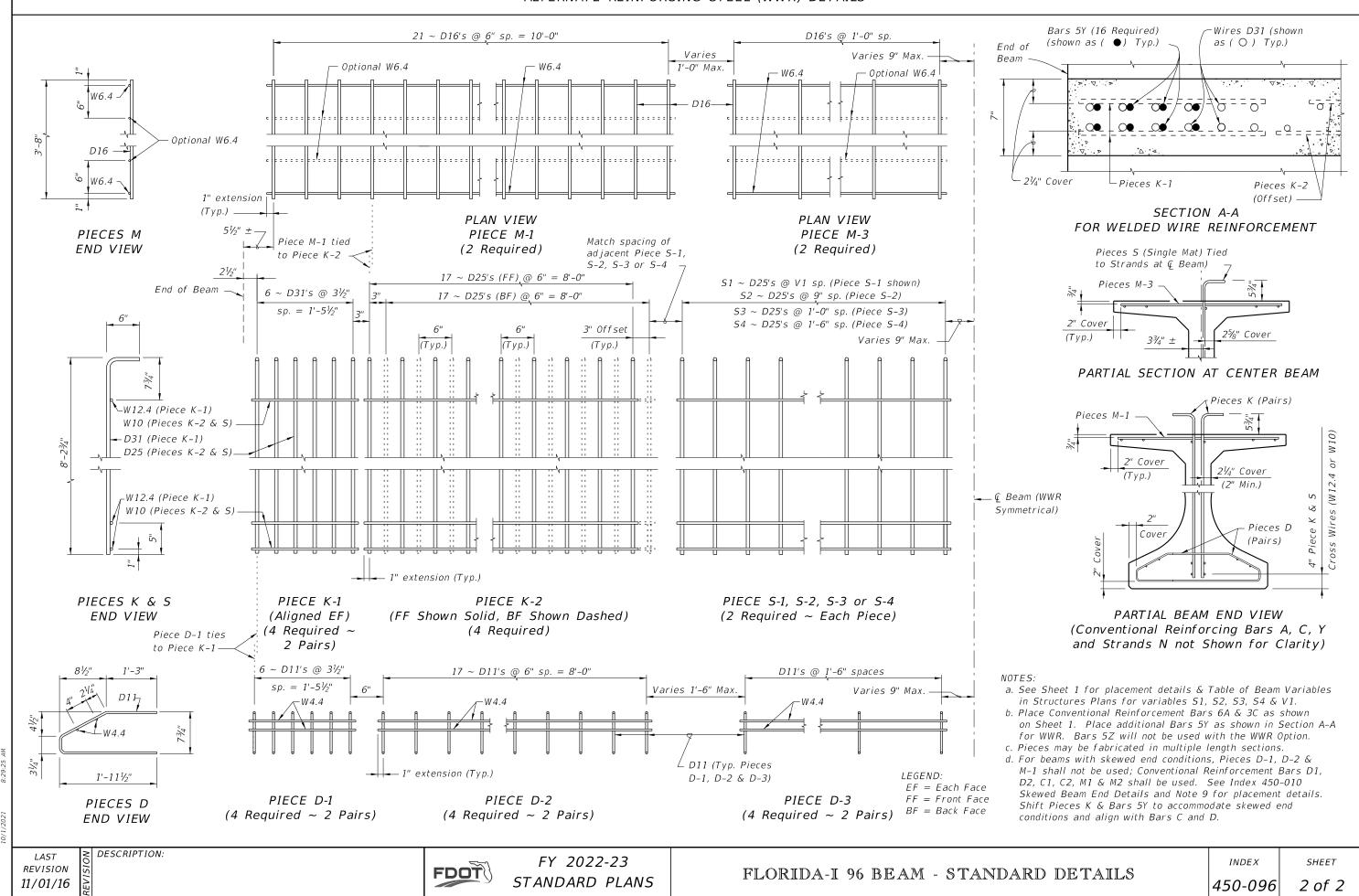


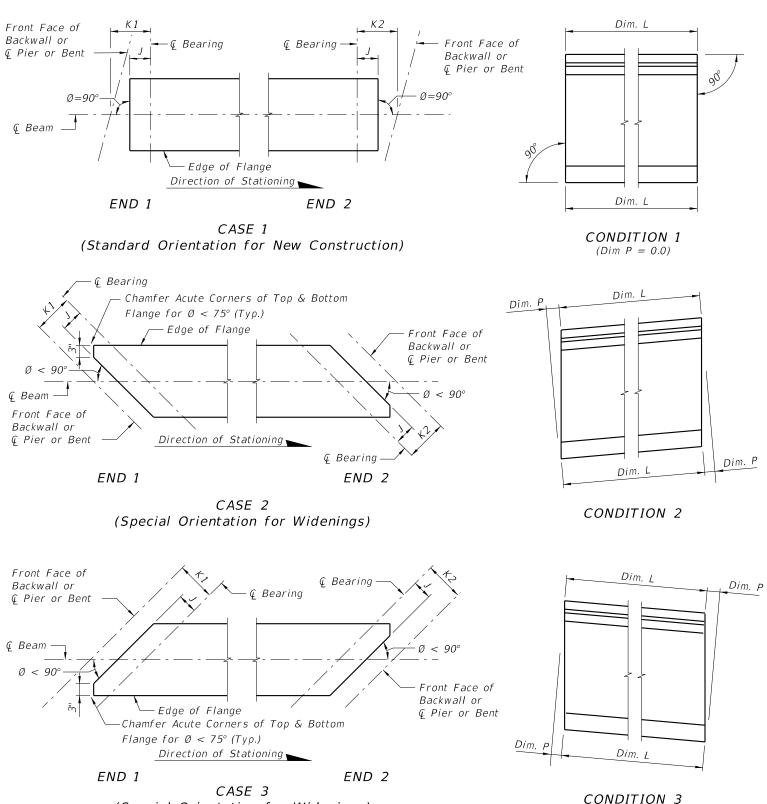












(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

BEAM NOTES

- 1. Work this Index with the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out to out.
- 3. Concrete cover: 2 inches minimum.
  - 4. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
  - 5. Place one (1) Bar 4K or 5Z at each location. Alternate the direction of the ends for each
  - 6. Tie Bars 4K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
  - 7. Place Bars 3D1 in beam END 1, and Bars 3D2 in beam END 2.
  - 8. For Beams with vertically beveled end conditions:
    - A. Place first row of Bars 3D1, 3D2, 4K, 4Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1".
    - For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
  - 9. For beams with skewed end conditions:
    - WWR is not permitted for end reinforcement Bars 3D1, and 3D2 on skewed ends; use bar reinforcement.
    - Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3D1, 3D2, 4K, 4Y and 5Z placed within the limits of the spacing for Bars 3D in "ELEVATION AT END OF BEAM".
    - Beyond the limits of the spacing for Bars 3D, place Bars 4K perpendicular to the longitudinal axis of the beam. For placement see "SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES" (Sheet 2).
  - 10. Contractor Options:
    - A. Deformed WWR may be used in lieu of Bars 3D, 4K, and 5Z as shown on Sheet 4; except at skewed ends (See Note 9).
    - Bars 3D1 and 3D2 may be fabricated as a two-piece bar with a 1'-0" minimum lap splice of the bottom legs.
  - For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands or above Strands N.
  - 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of required anchorage devices.
  - 12. For beams with ends that will not to be encased in concrete diaphragms, cut wedges and recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2.
  - 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
  - The superstructure environmental classification is slightly or moderately aggressive
  - Clear cover to adjacent steel reinforcing is 1"or greater
  - Hole inside diameter is 2" maximum
  - Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

DETAILS AND NOTES

REVISION 11/01/18

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

SCHEMATIC END ELEVATIONS OF BEAMS (Showing Vertical Bevel of Beam End)

AASHTO TYPE II BEAM

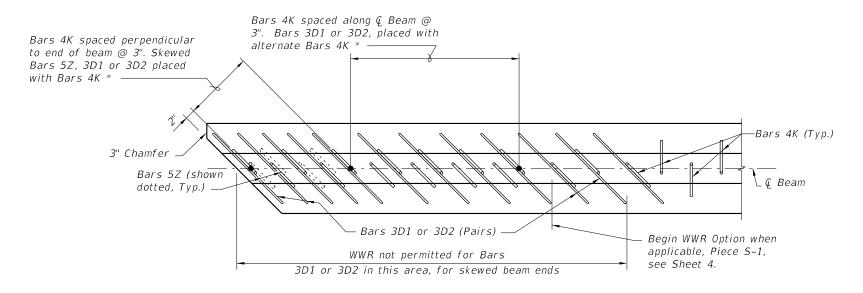
INDEX 450-120

SHEET

1 of 4

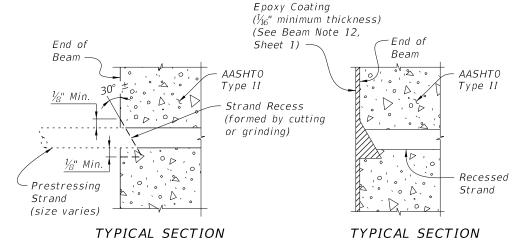
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

\* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for WWR.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE) (End 1 Shown, End 2 Similar) (Bars 4Y & Strands not shown for clarity)

= SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



SHOWING CUT STRAND RECESS LIMITS

AFTER PROTECTING

=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

**REVISION** 11/01/19

DESCRIPTION:



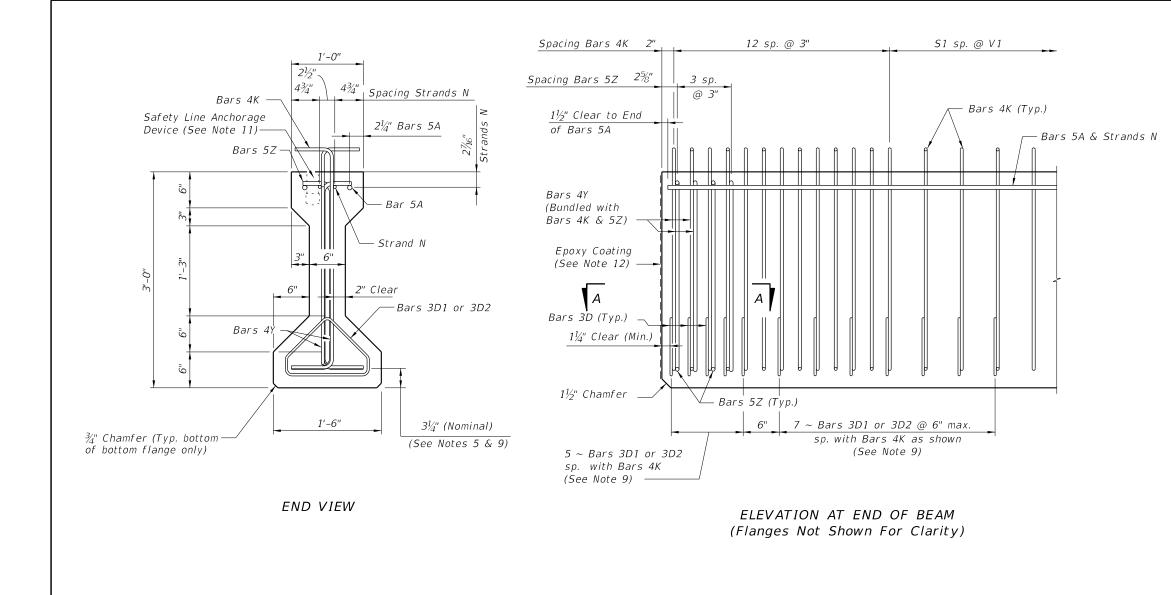
FY 2022-23 STANDARD PLANS

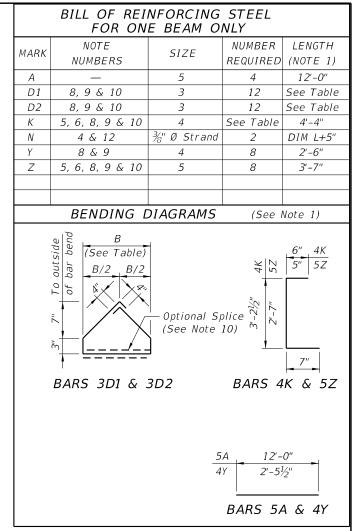
AASHTO TYPE II BEAM

INDEX

SHEET

450-120 2 of 4

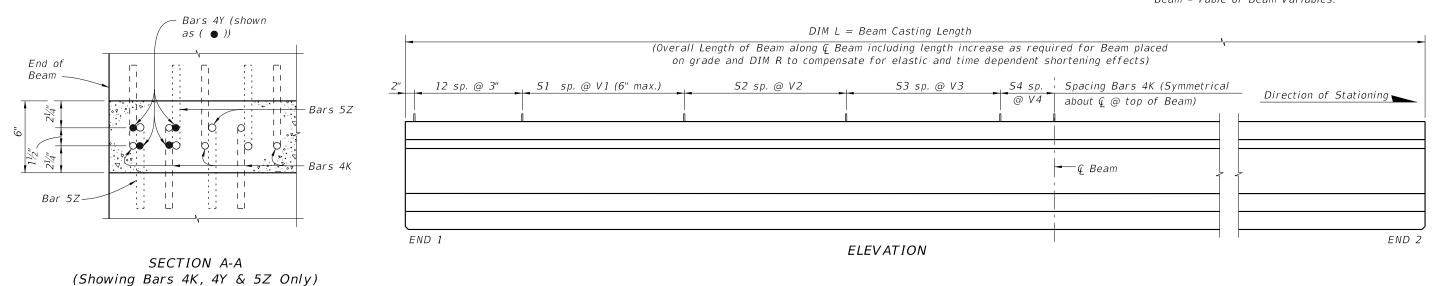




Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.



11/01/19

DESCRIPTION:

FDOT

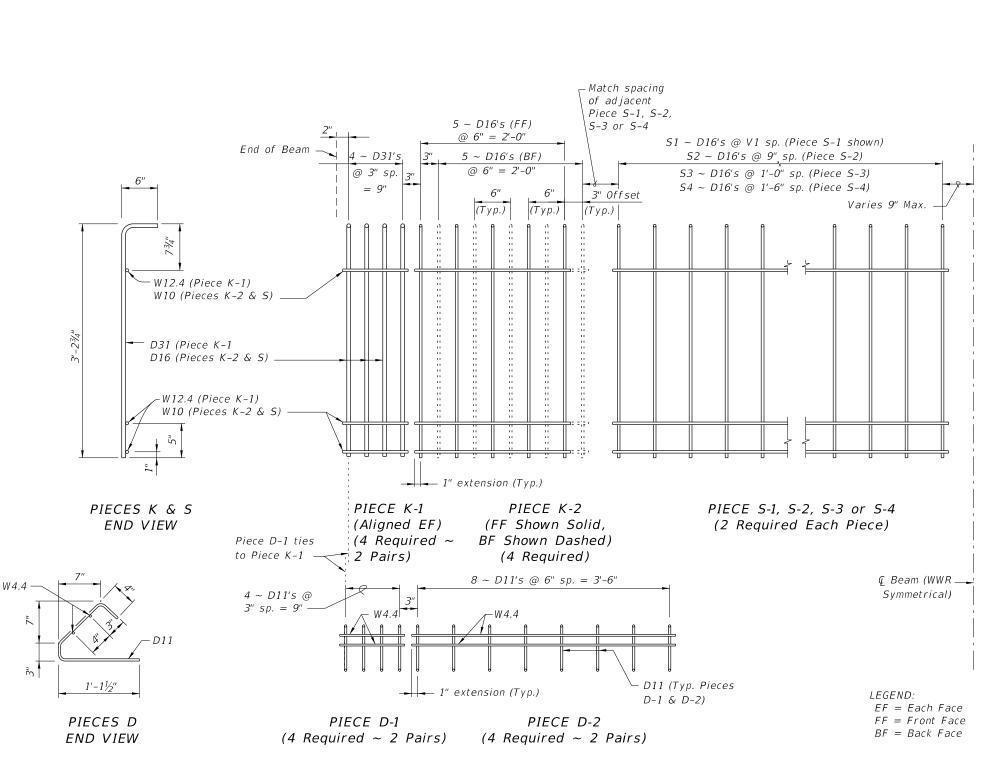
FY 2022-23 STANDARD PLANS

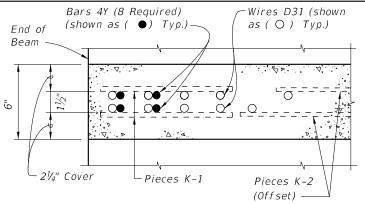
AASHTO TYPE II BEAM

INDEX SHEET

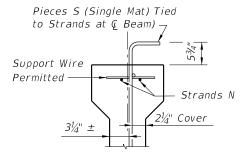
STANDARD DETAILS

450-120 3 of 4

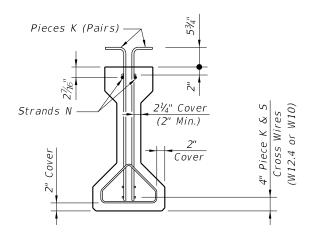




# SECTION A-A FOR WELDED WIRE REINFORCEMENT



# PARTIAL SECTION AT CENTER BEAM



PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

#### NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, S4 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

STANDARD DETAILS

LAST REVISION 11/01/16

DESCRIPTION:

FDOT

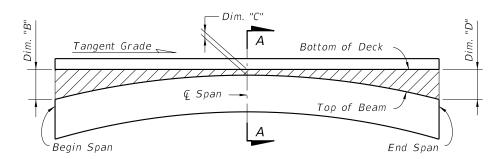
FY 2022-23 STANDARD PLANS

AASHTO TYPE II BEAM

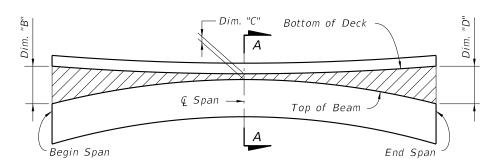
INDEX

SHEET

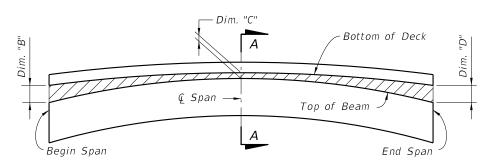
450-120 4 of 4



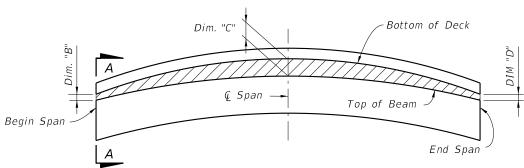
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT Q SPAN
(ALONG Q BEAM) (CASE 3)

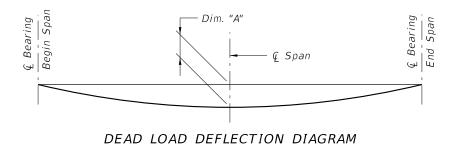


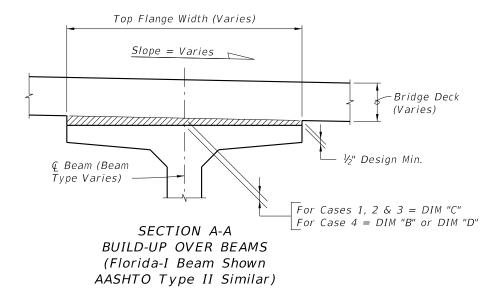
BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS
- CONTROL AT BEGIN OR END SPAN
(ALONG Q BEAM) (CASE 4)

#### BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table\* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table\*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



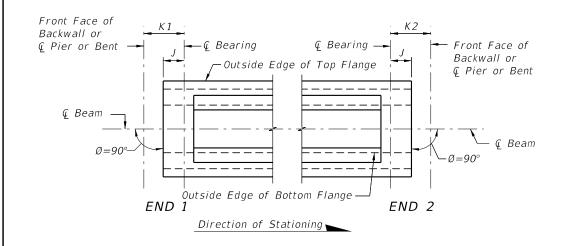


\* NOTE:

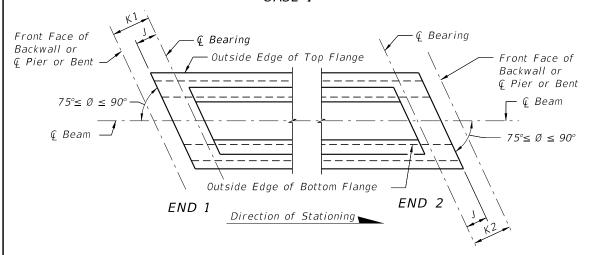
Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.

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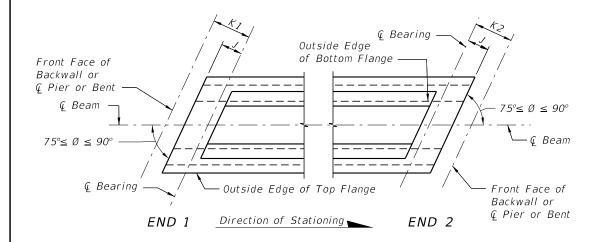
DESCRIPTION:



### CASE 1



### CASE 2



CASE 3

SCHEMATIC PLAN VIEWS AT BEAM ENDS =

# REVISION 11/01/16

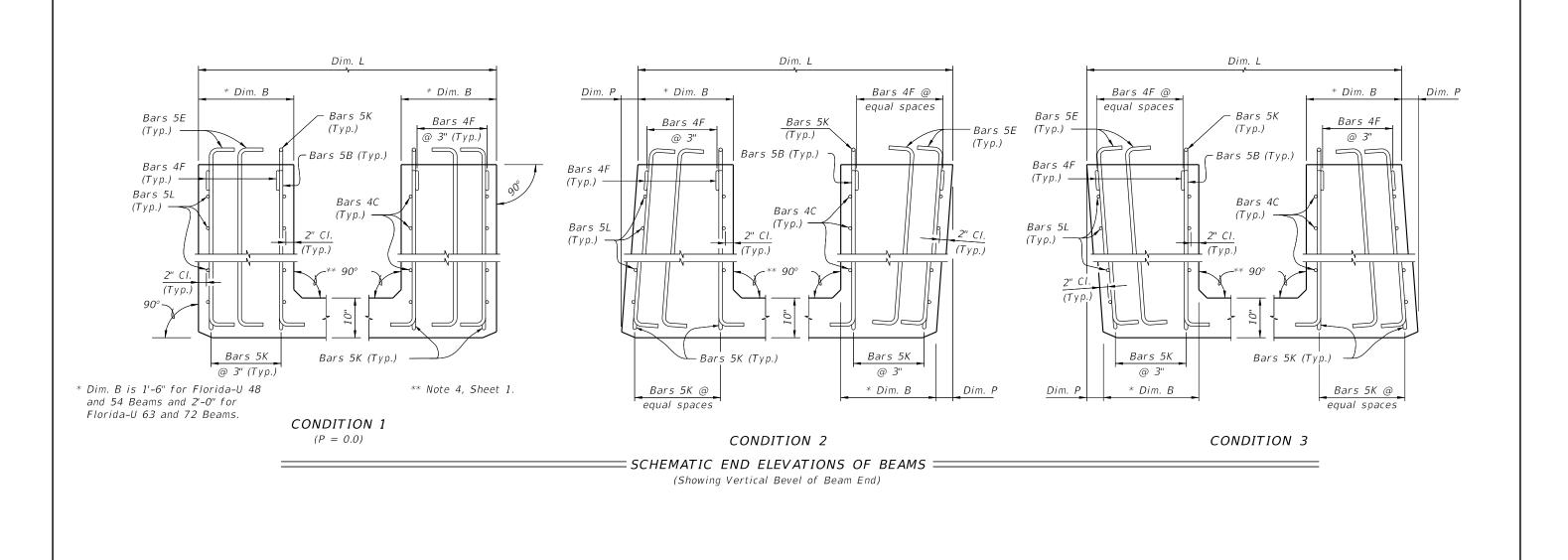
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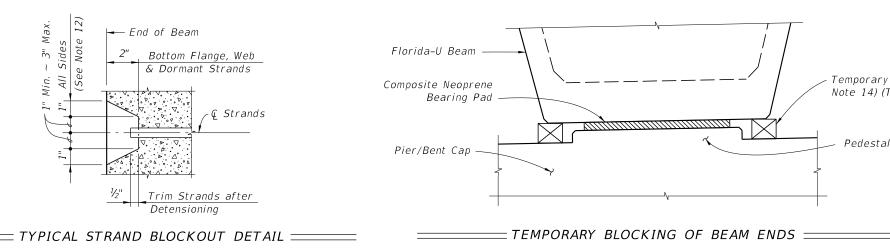


FY 2022-23 STANDARD PLANS

#### BEAM NOTES

- 1. Work this Index with the Florida-U Beam Standard Details (Index 450-248, 450-254, 450-263 and 450-272) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum. Maximum aggregate size is a No. 67.
- 4. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 5. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 6. Tie Bars 5K to the fully bonded strands in the bottom row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. For beams without skewed ends or vertically beveled end conditions (see Note 8) the Engineer may approve the use of deformed WWR in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M. The spacing and sizes of deformed WWR must match the reinforcing sizes shown on the Florida-U Beam Standard Details sheets.
- 8. For Beams with vertically beveled end conditions, where "Dim. P" exceeds 1", place Bars 5E, and the first Bars 4F and 5K parallel to the end of the beam. Fan the remaining Bars 4F and 5K within the limits of "Dim. B" (End Diaphragm) at equal spaces until vertical.
- 9. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any anchorage devices or other required embedded hardware.
- 10. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing the beam from casting bed.
- 11. Place drains pipes adjacent to each web at each beam end (four drains per beam).
  - Drain Pipe: 2" NPS Schedule 80 PVC.
  - Cover, wrap and secure wire screen around the end of the pipe prior to casting. Extend screen a minimum of 1" down the pipe sides.
  - Provide removable pipe plugs during casting. Remove plugs from the inside of
- 12. Protection of Strands:
  - A. Provide a 2" deep recess around all strands (including dormant) or strand groups. Extend the recessed blockout to the web face and bottom of the flange for the bottom row of strands.
  - After detensioning, cut strands  $\frac{1}{2}$ " from recessed surface and fill the blockout to protect strands with Type F-2 or Q Epoxy Compound in accordance with Specification Section 926.
- 13. Use Stay-In-Place metal deck forms inside the beams.
- 14. Prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations during deck placement. Leave temporary blocking and bracing in place for a minimum of four days after the deck is placed.
- 15. Based on the deck forming system and deck placement sequence, evaluate and provide any required temporary bracing between the U Beams.





REVISION 11/01/16

DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

FLORIDA-U BEAM - TYPICAL DETAILS & NOTES

Temporary Blocking (See

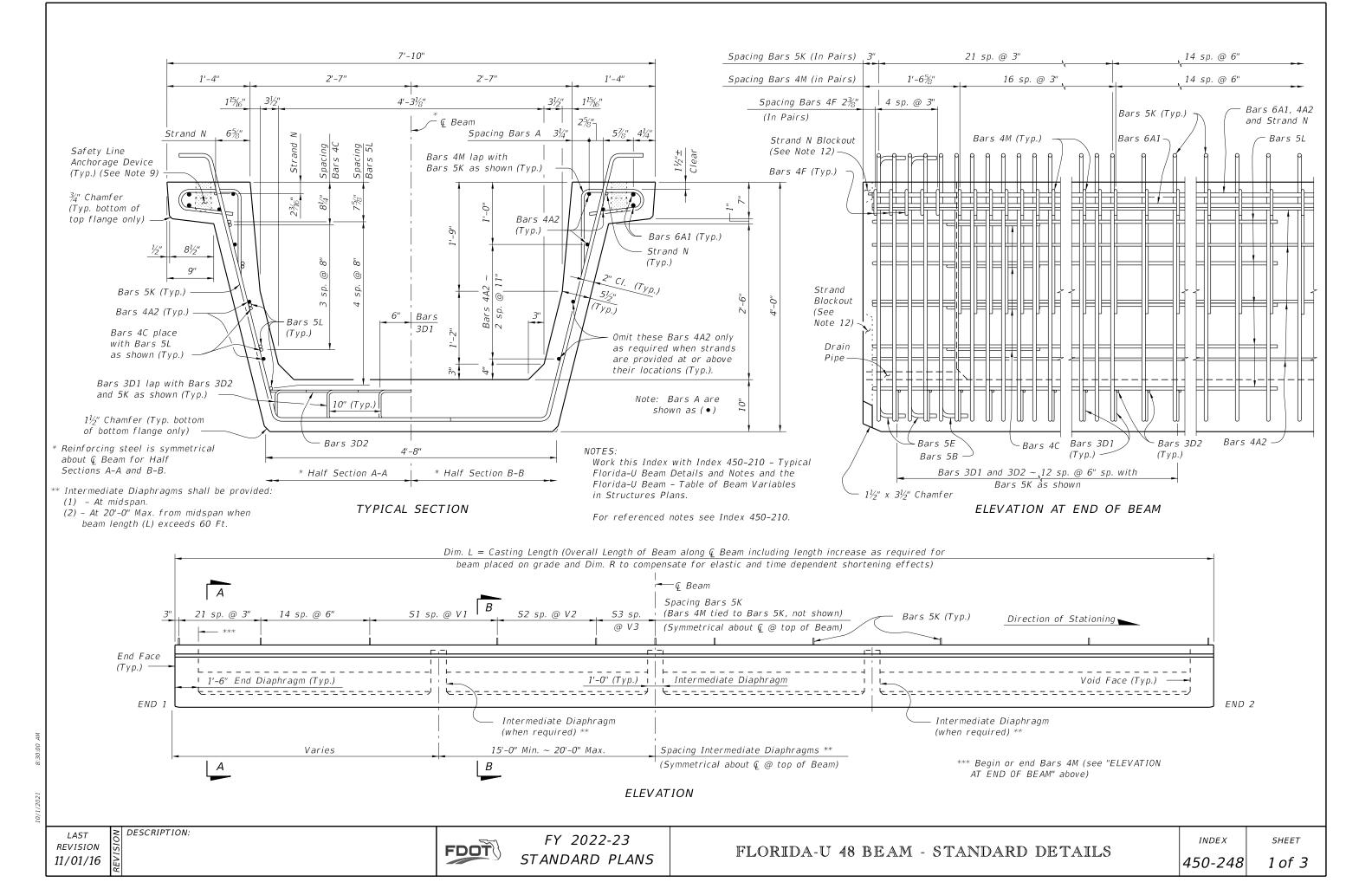
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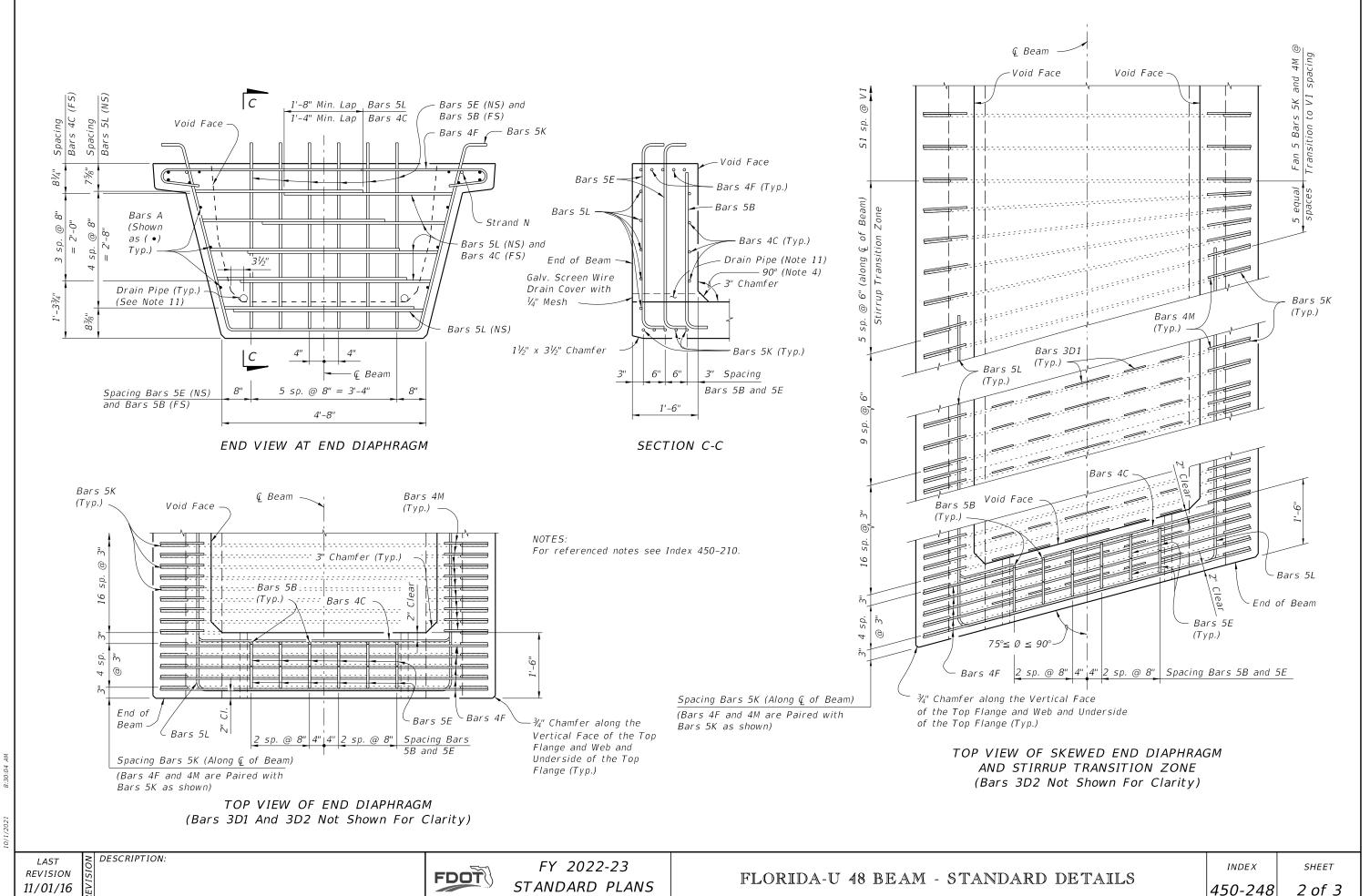
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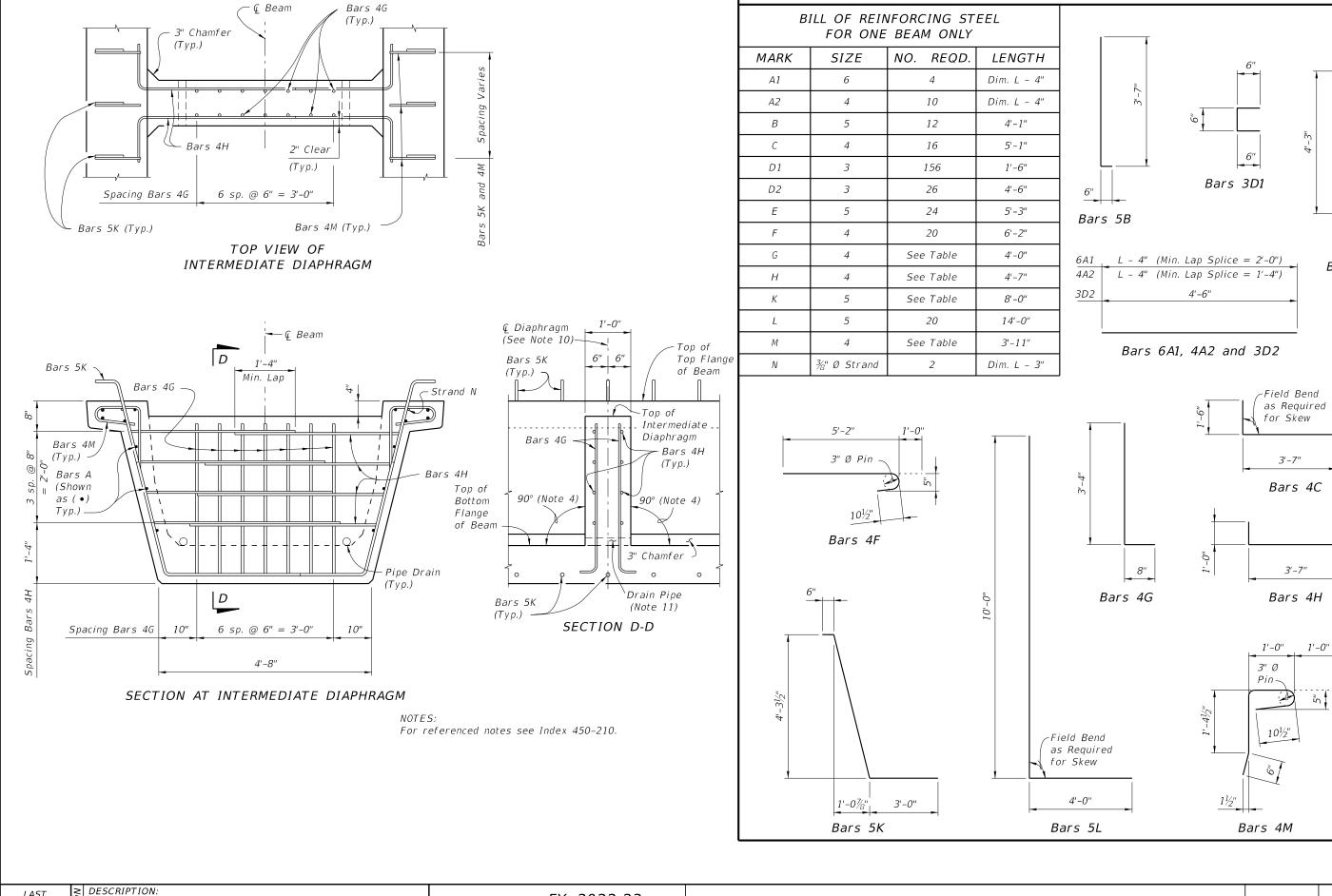
INDEX 450-210

SHEET

2 of 2







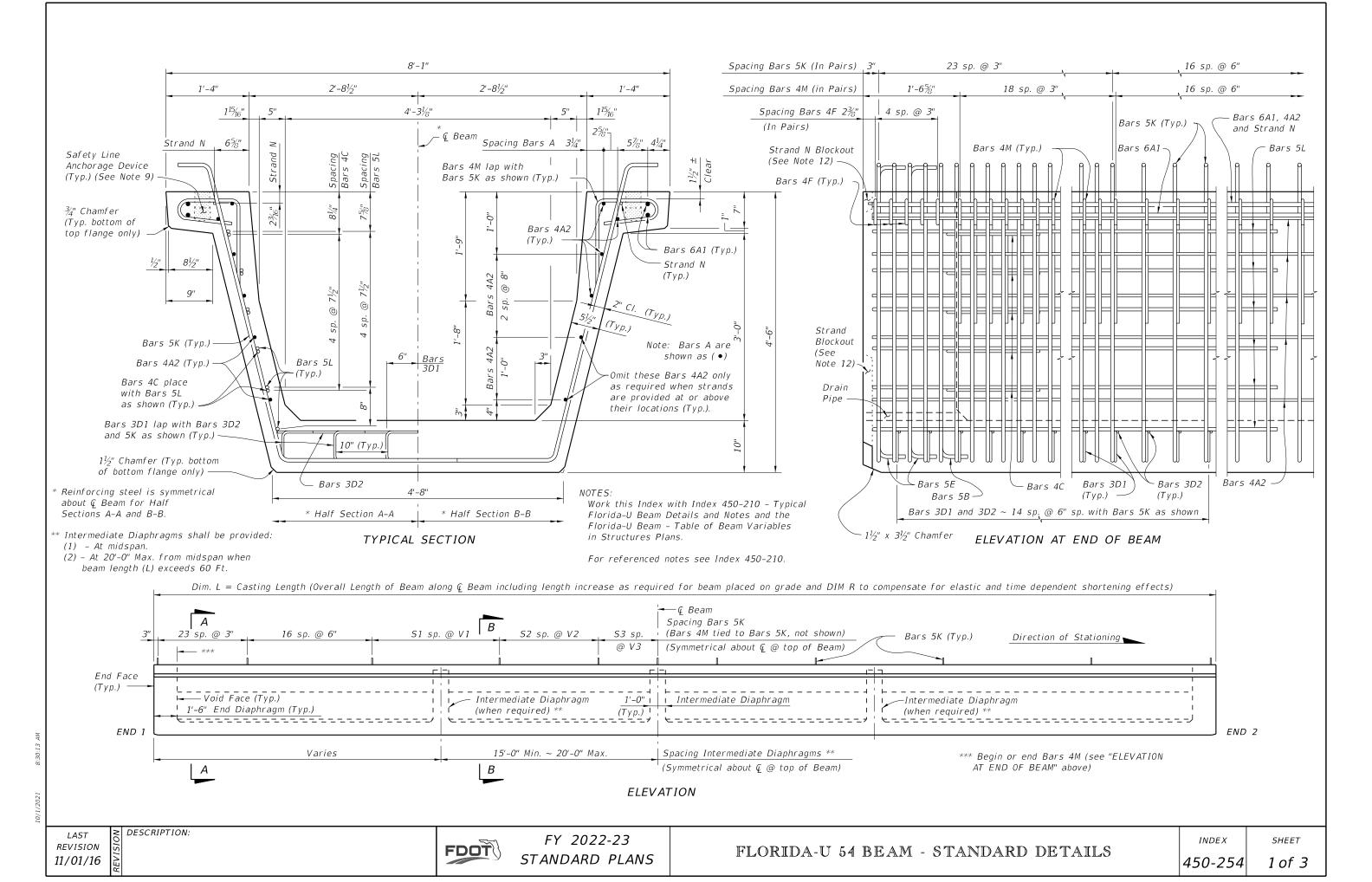
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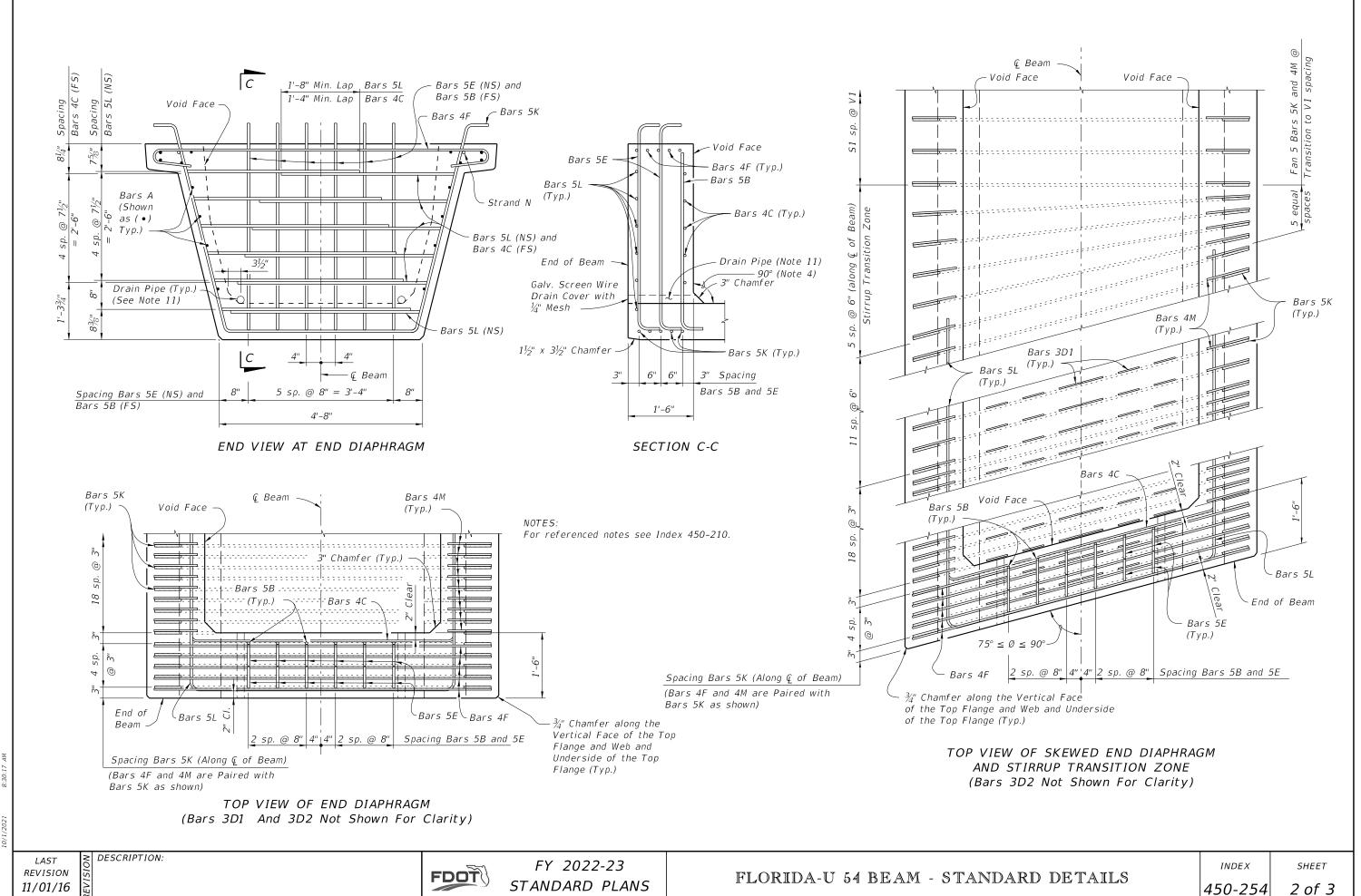
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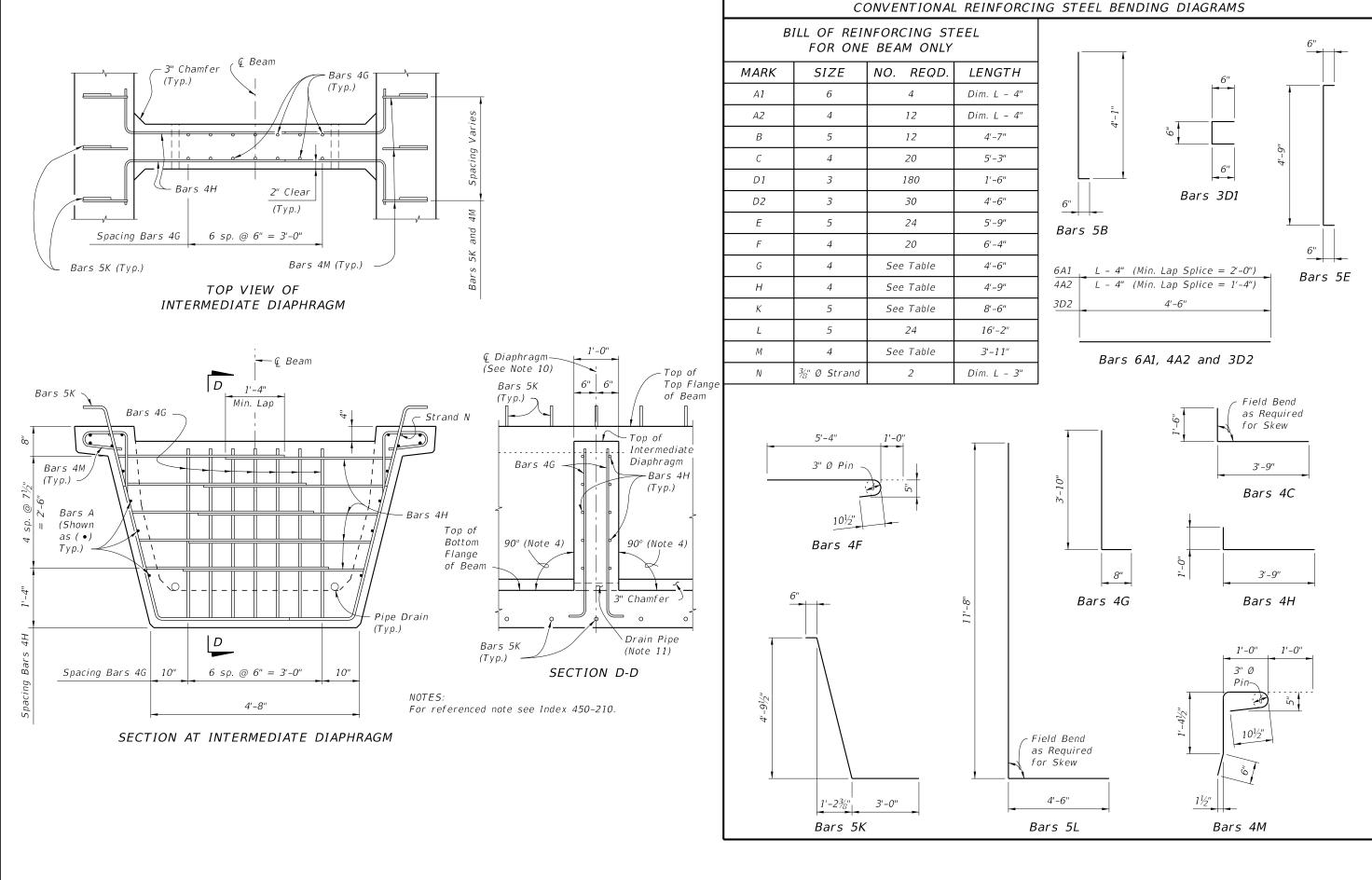
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FY 2022-23 STANDARD PLANS CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

Bars 5E



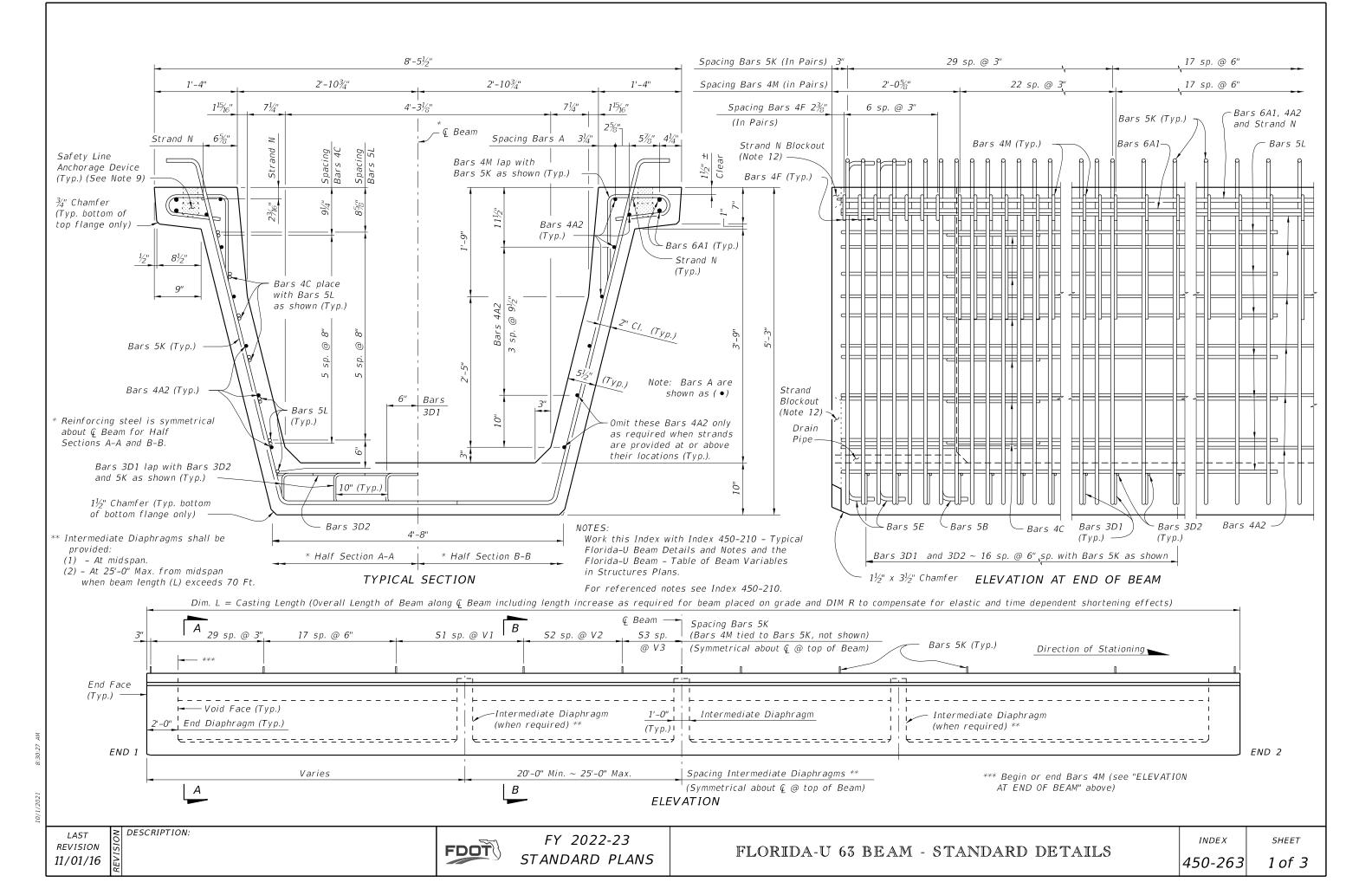


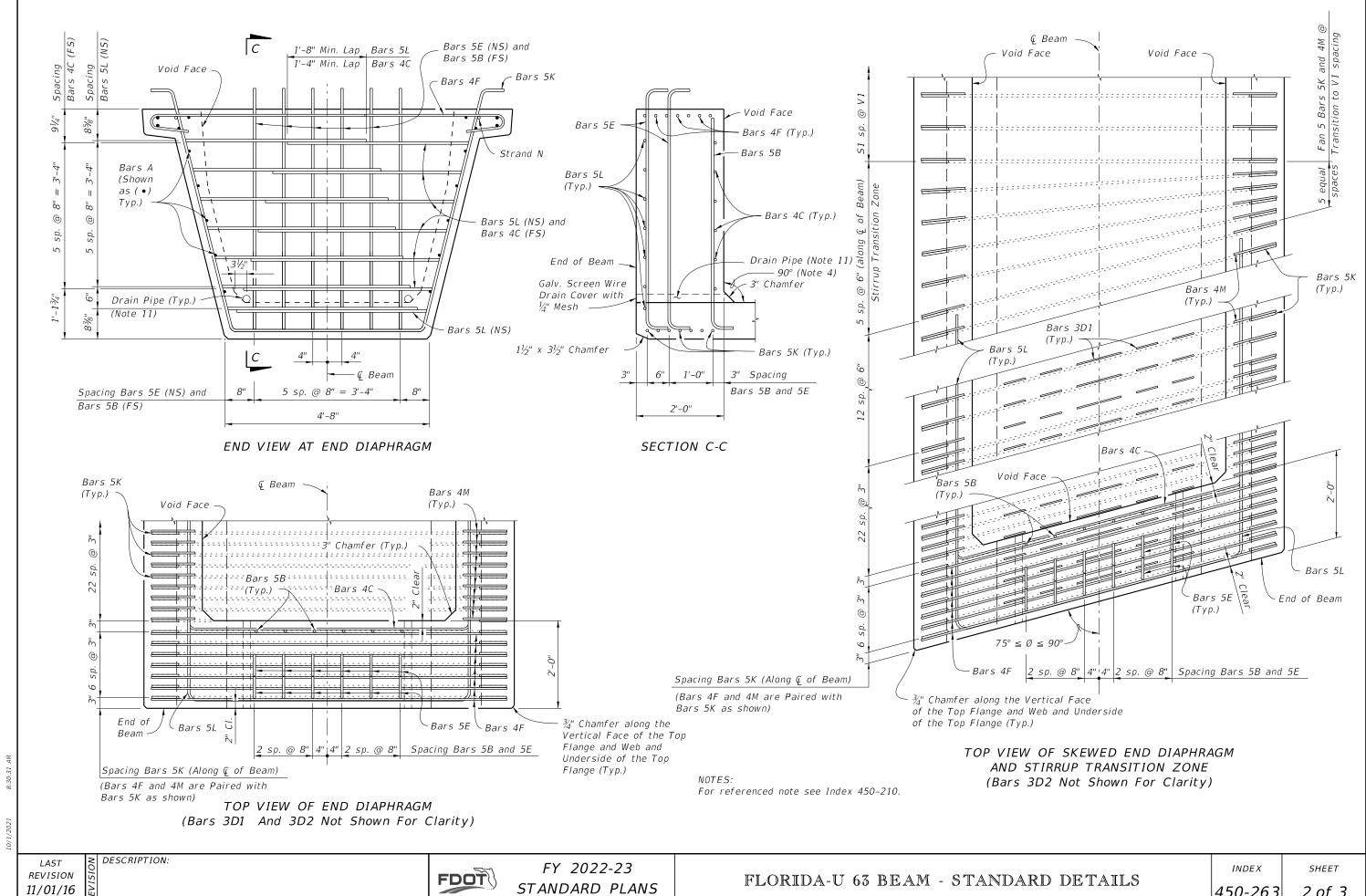


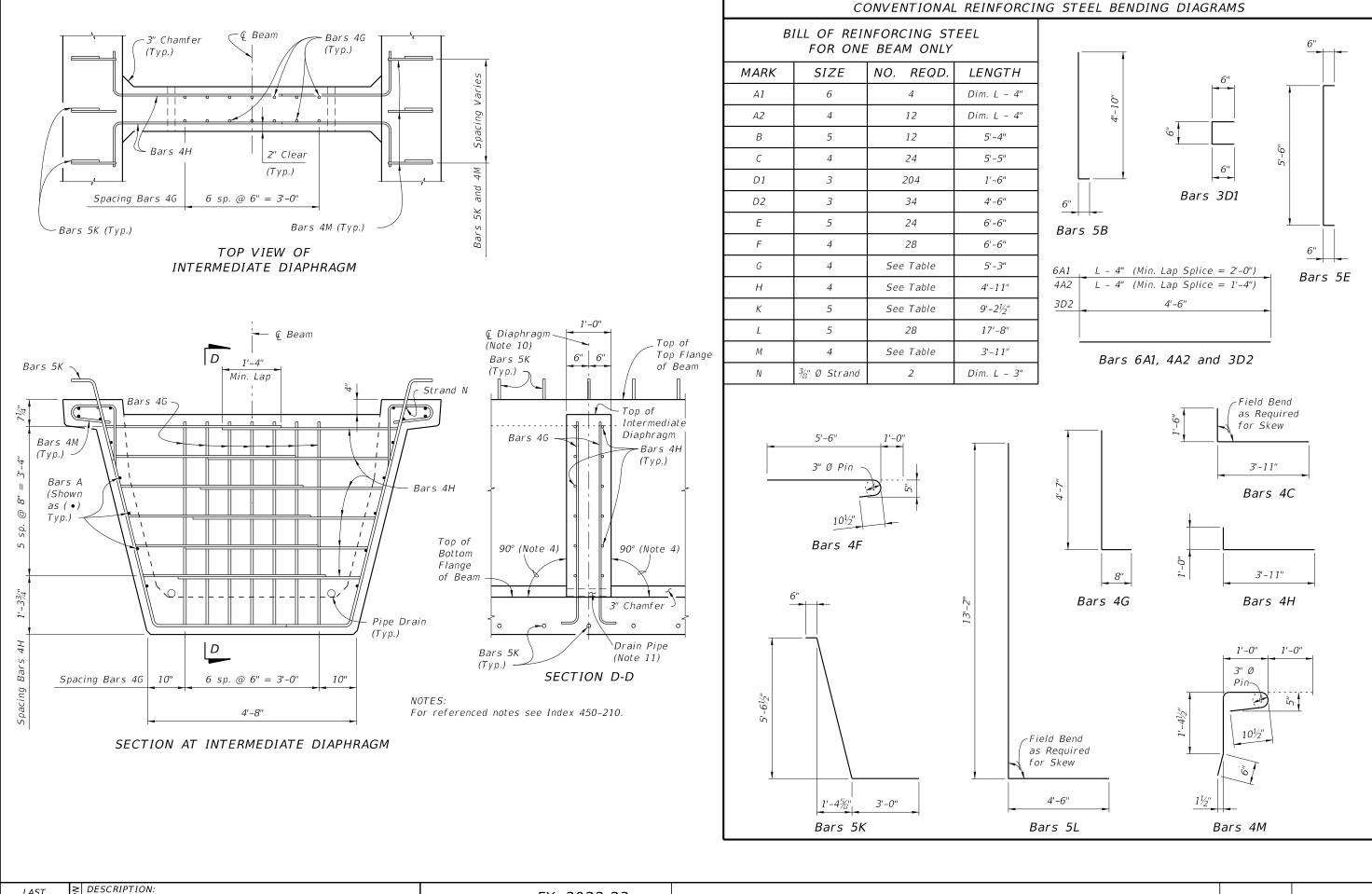
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LAST REVISION 11/01/16

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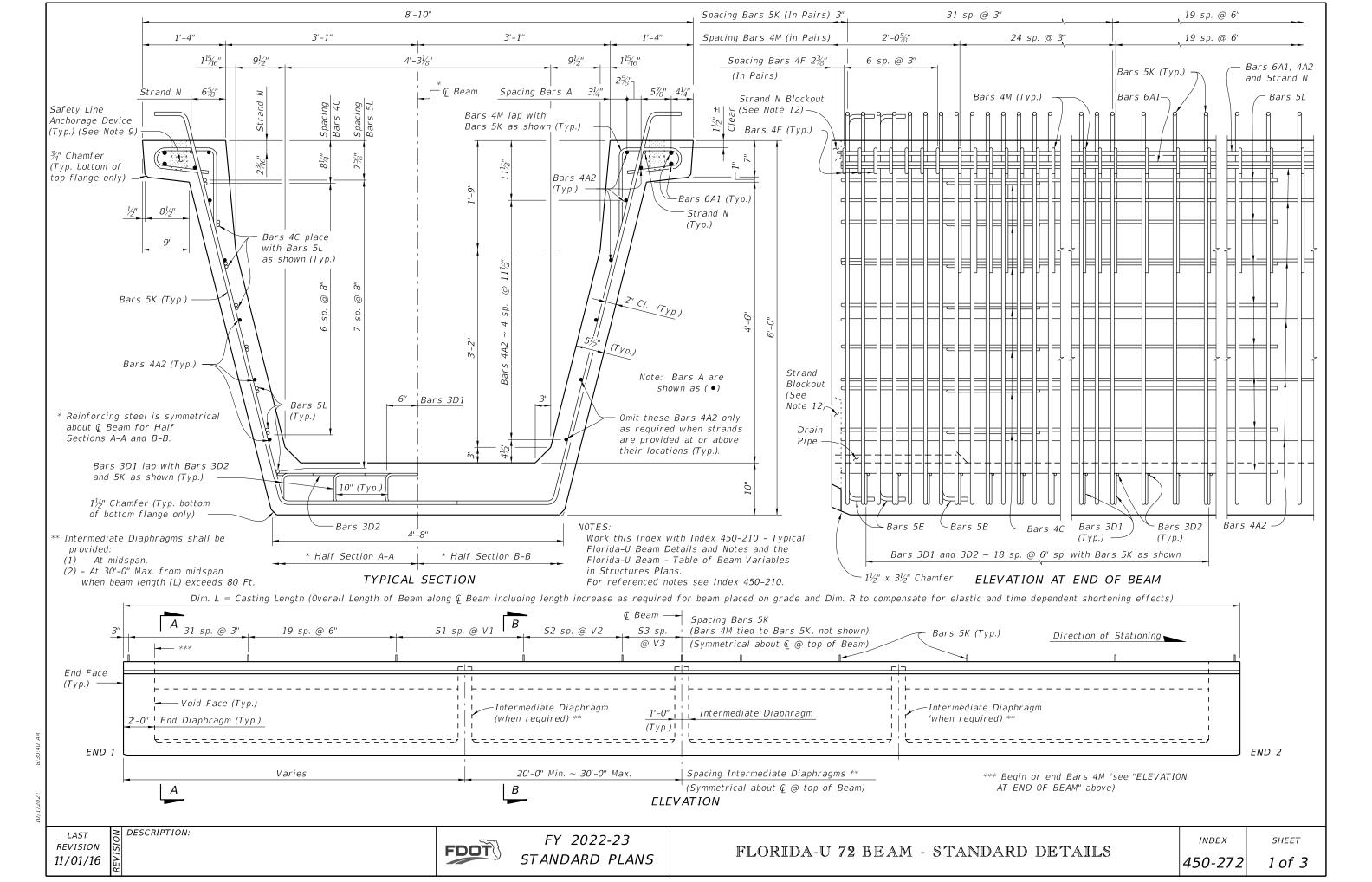


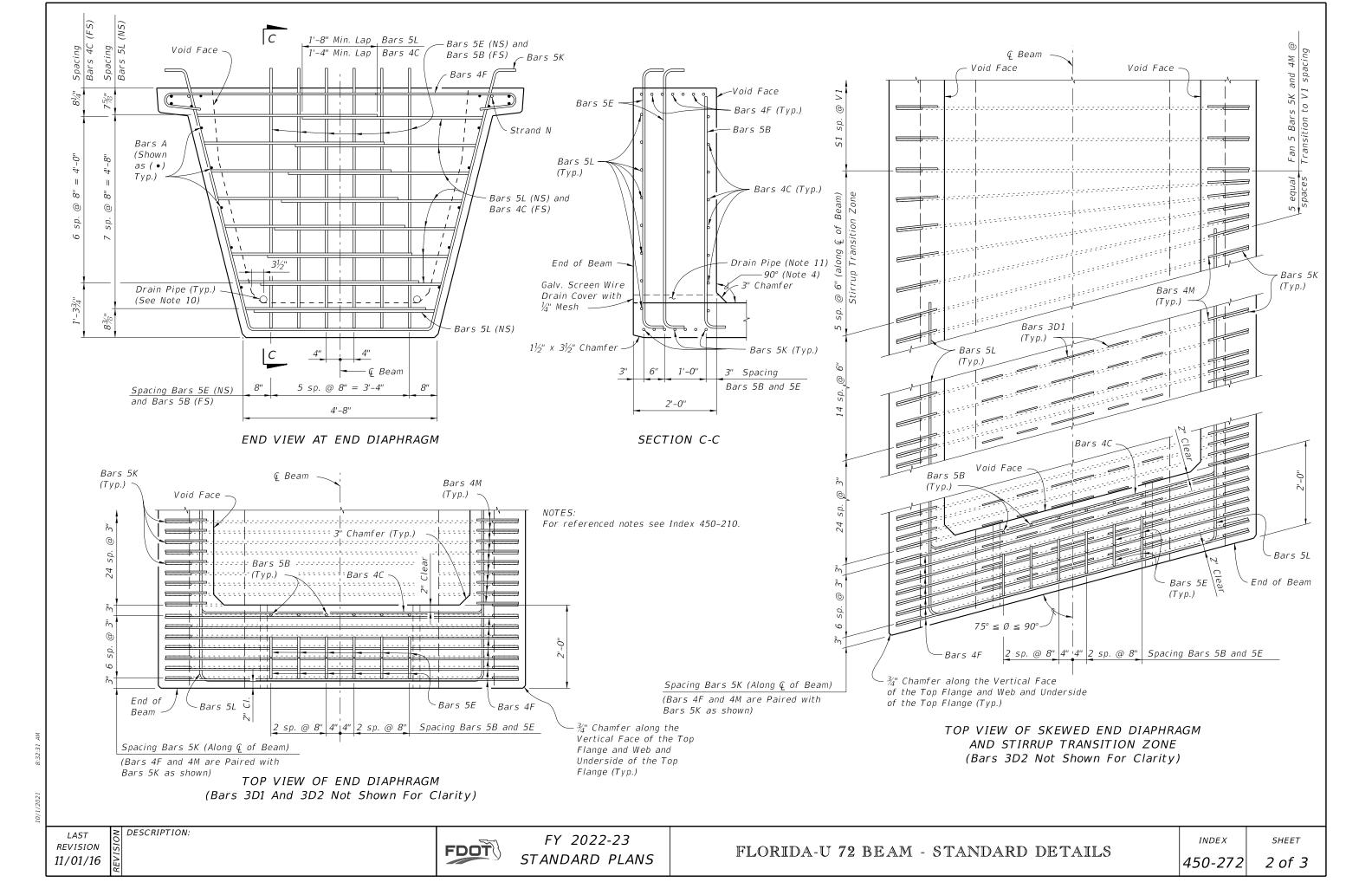


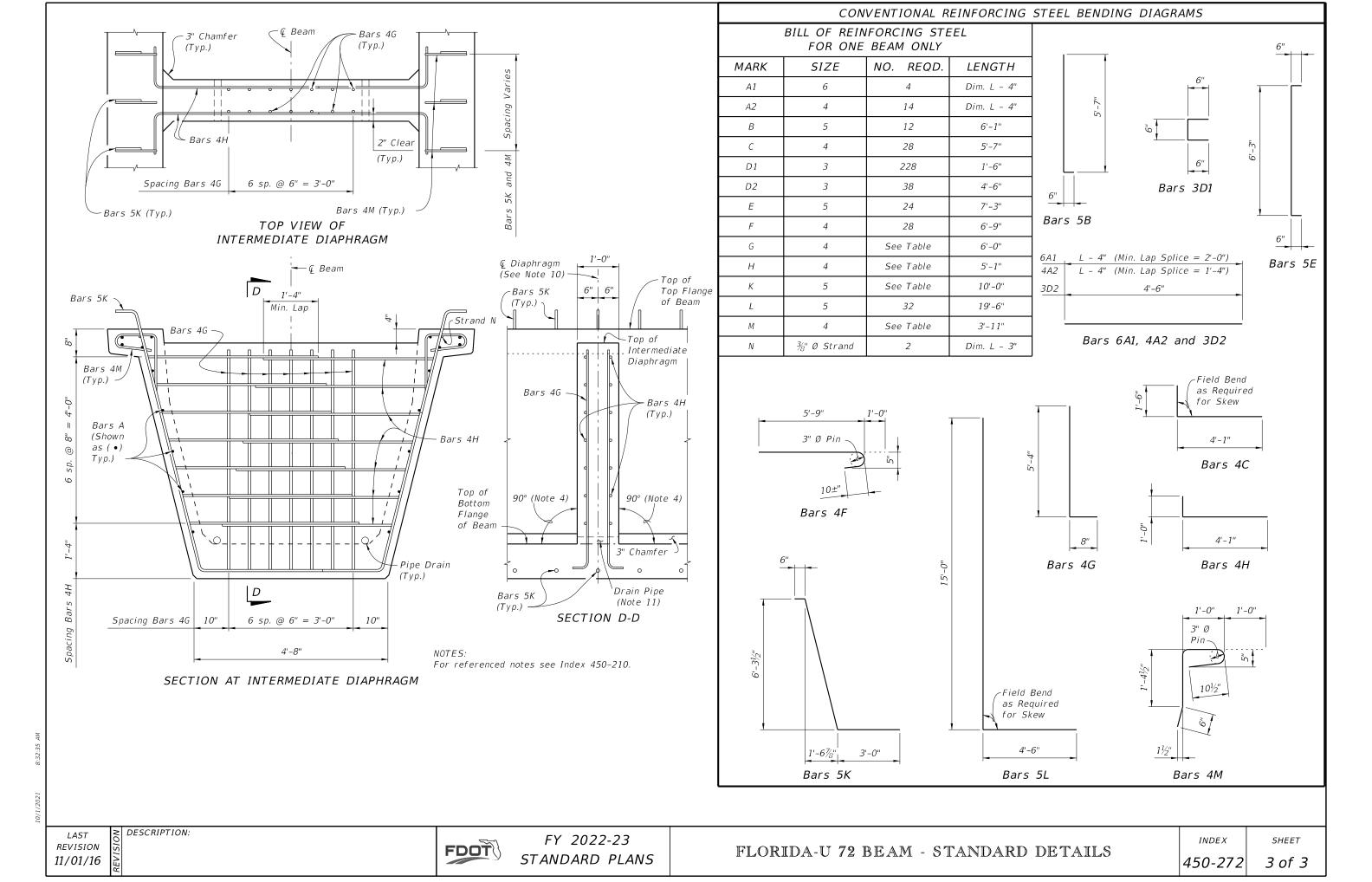


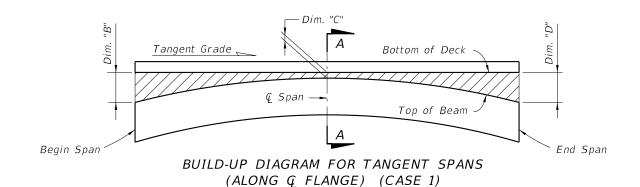
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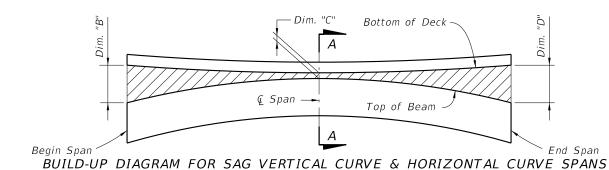
LAST REVISION 11/01/16

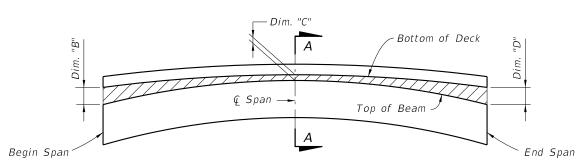






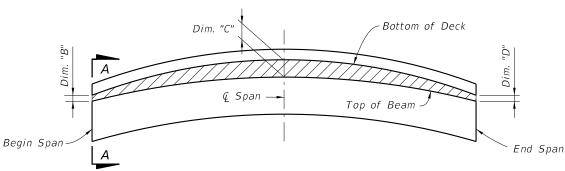






(ALONG Q FLANGE) (CASE 2)

BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG G FLANGE) (CASE 3)

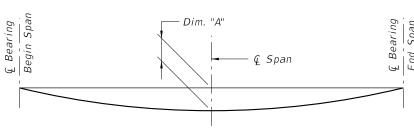


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG G FLANGE) (CASE 4)

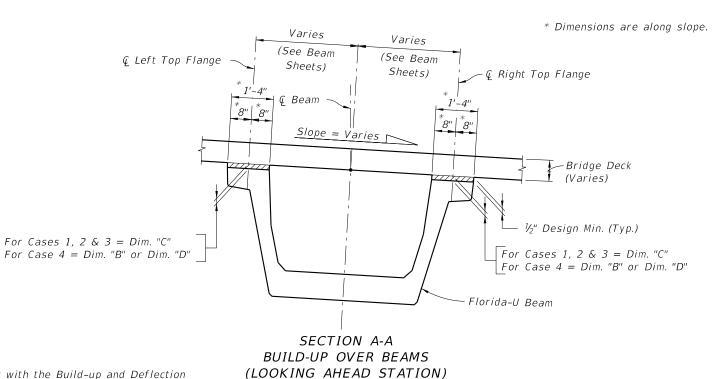
#### BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table\* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table\*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM (ALONG G BEAM)



Work this Index with the Build-up and Deflection Data Table for Florida-U Beams in Structures Plans.

FLORIDA-U BEAMS

- BUILD-UP & DEFLECTION DATA

INDEX

SHEET

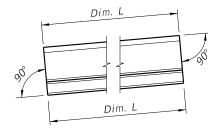
**REVISION** 

FDOT

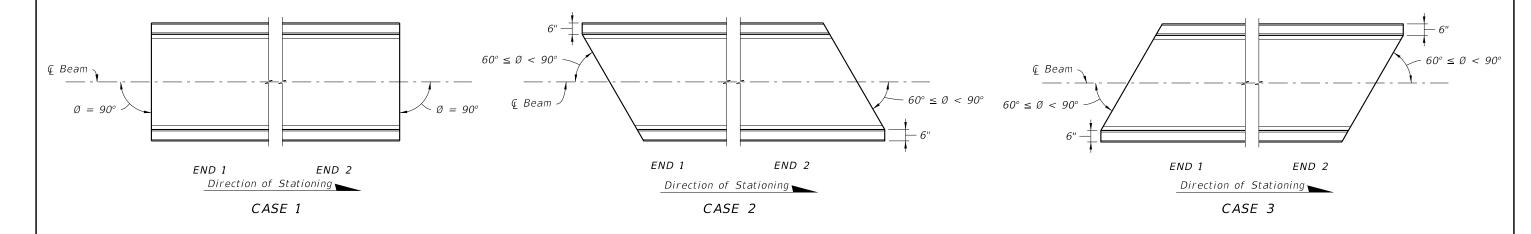
FY 2022-23 STANDARD PLANS

#### FABRICATION NOTES

- 1. The abbreviated FSB designation for depth and width is FSB "depth" x "width", e.g. FSB 12 x 48.
- 2. All bar dimensions are out-to-out.
- 3. Strands N shall be ASTM A416, Grade 250 or 270, 3/8" Ø or larger strands, stressed to 10,000 lbs. each.
- 4. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 5. For referenced Dimensions, Angles and Case Numbers, see Florida Slab Beam - Table of Variables in Structures Plans.
- 6. Bars 4D1 & 6Y1 correspond to END 1, and 4D2 & 6Y2 correspond to END 2.
- 7. Bars 5E1 correspond to interior FSBs, and 5E2 correspond to exterior FSBs.
- 8. Rake the top surface of the Slab Beams transversely to provide a roughened surface with  $\frac{1}{4}$ " amplitude.
- 9. Embedment of Safety Line Anchorage Devices are permitted to accommodate full protection systems. See shop drawings for details and spacings.



SCHEMATIC SIDE ELEVATION OF BEAM (Beam on a Positive Grade shown; Beam on a Negative Grade or Horizontal Grade similar.)

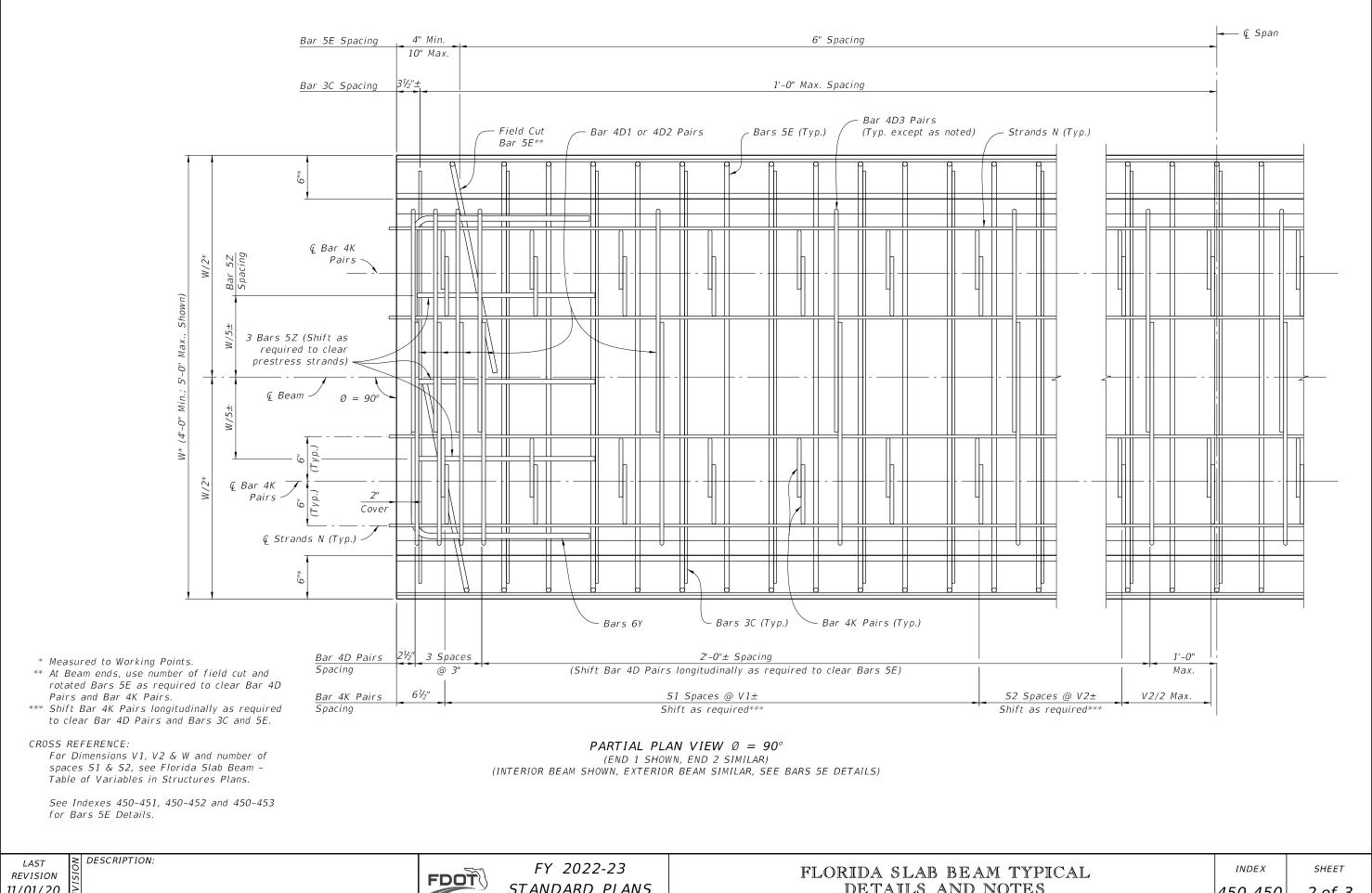


SCHEMATIC PLAN VIEWS AT BEAM ENDS

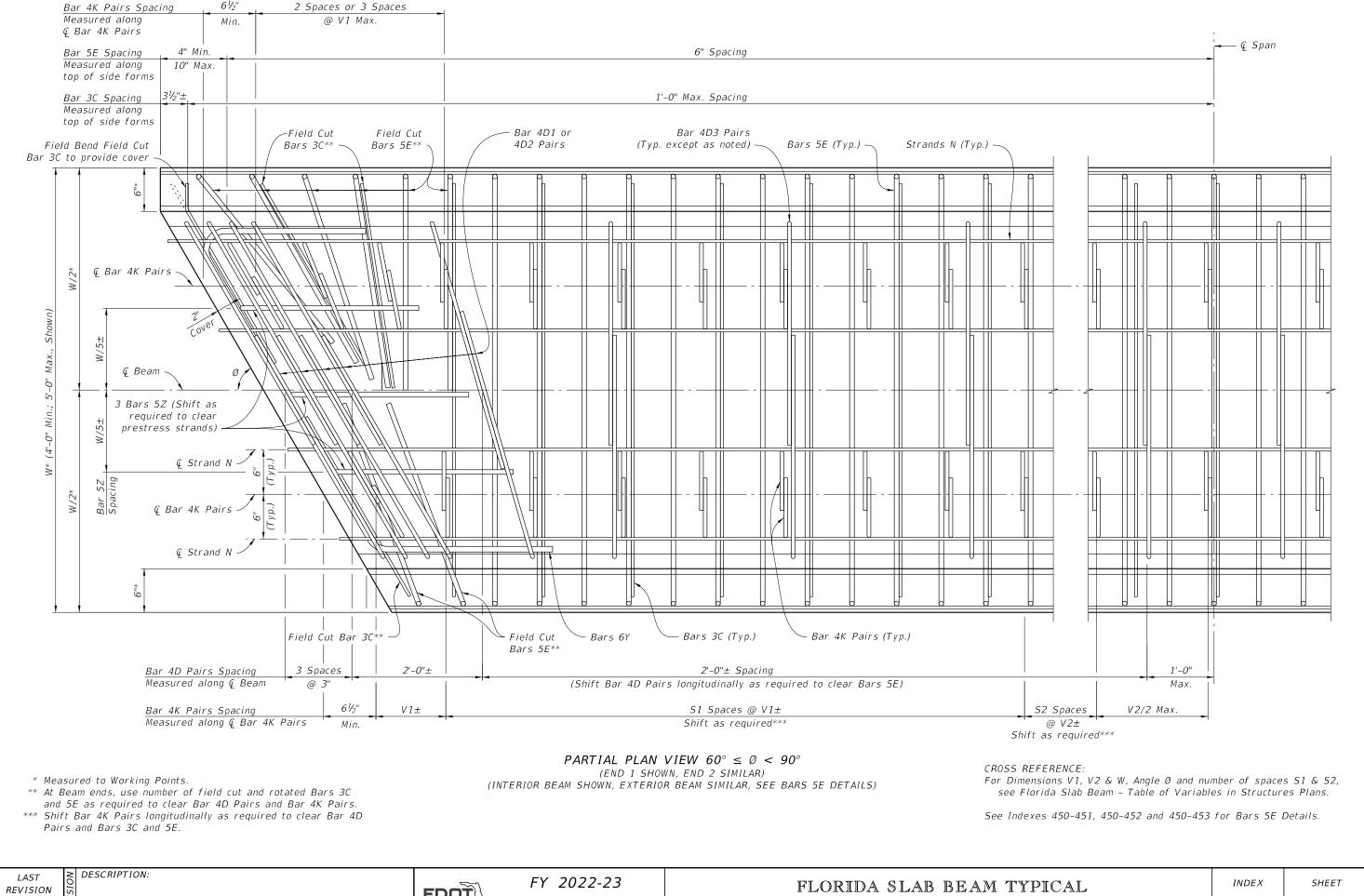
REVISION 11/01/20

DESCRIPTION:

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11/01/20

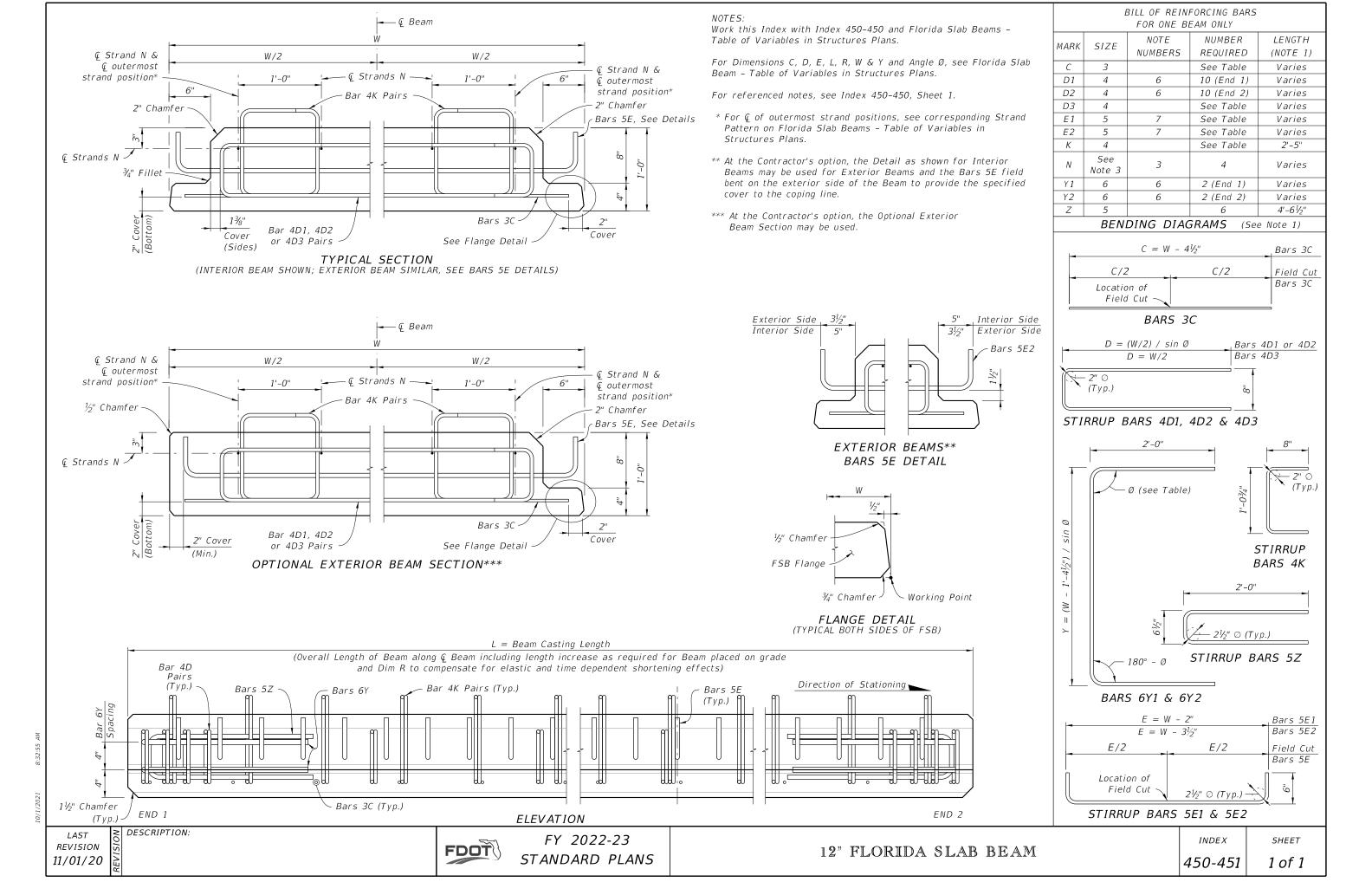


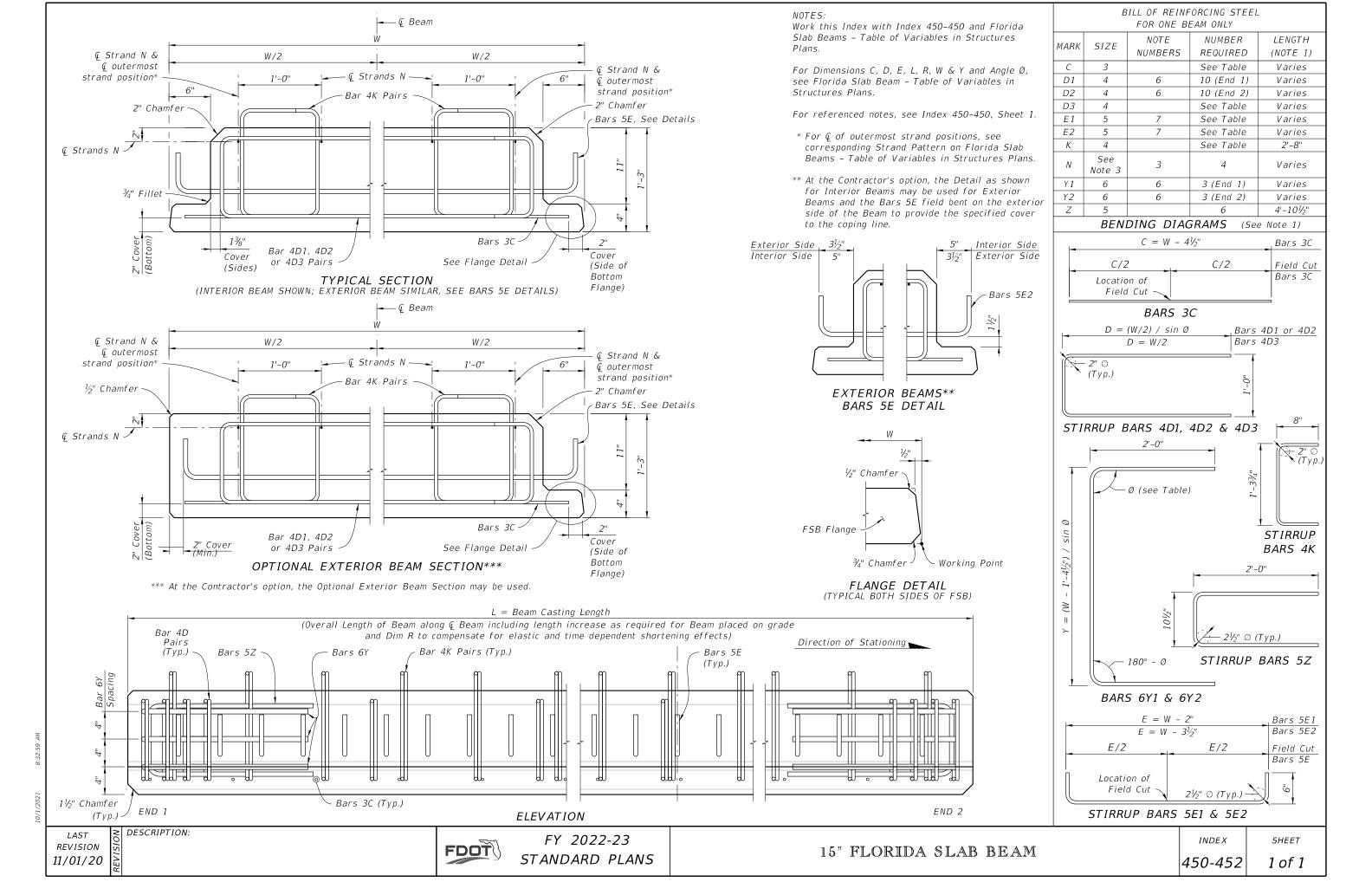
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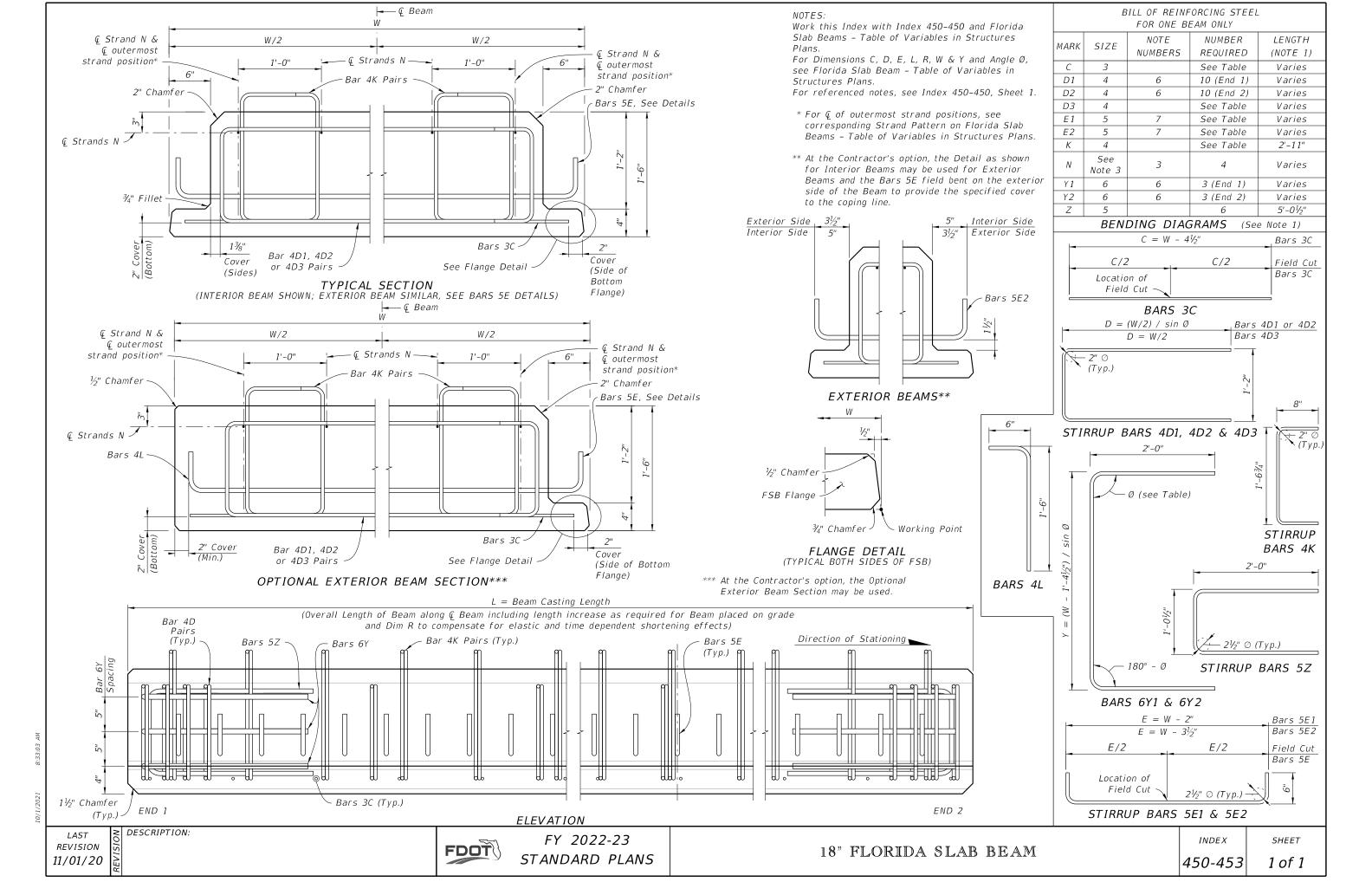
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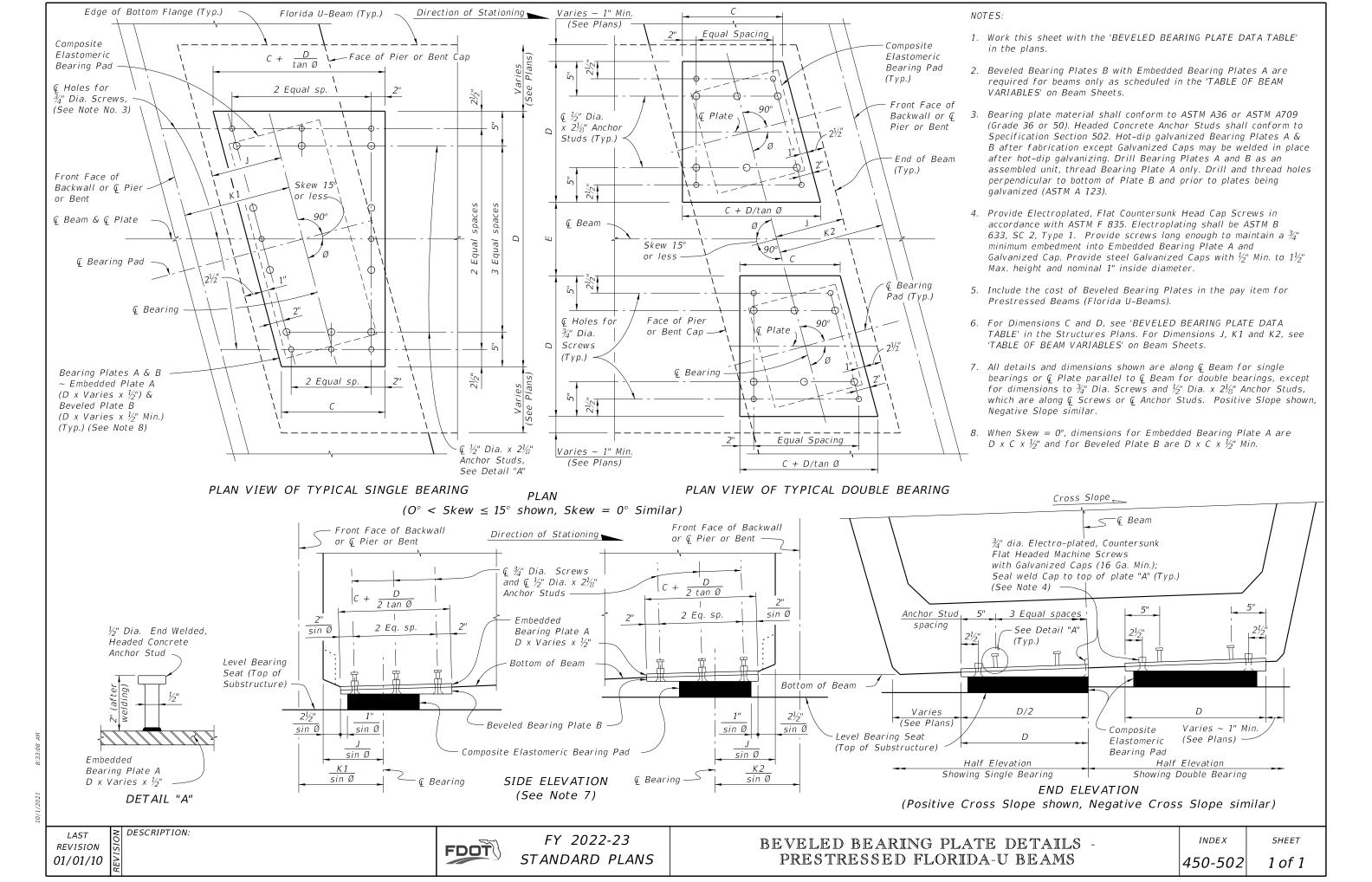
450-450

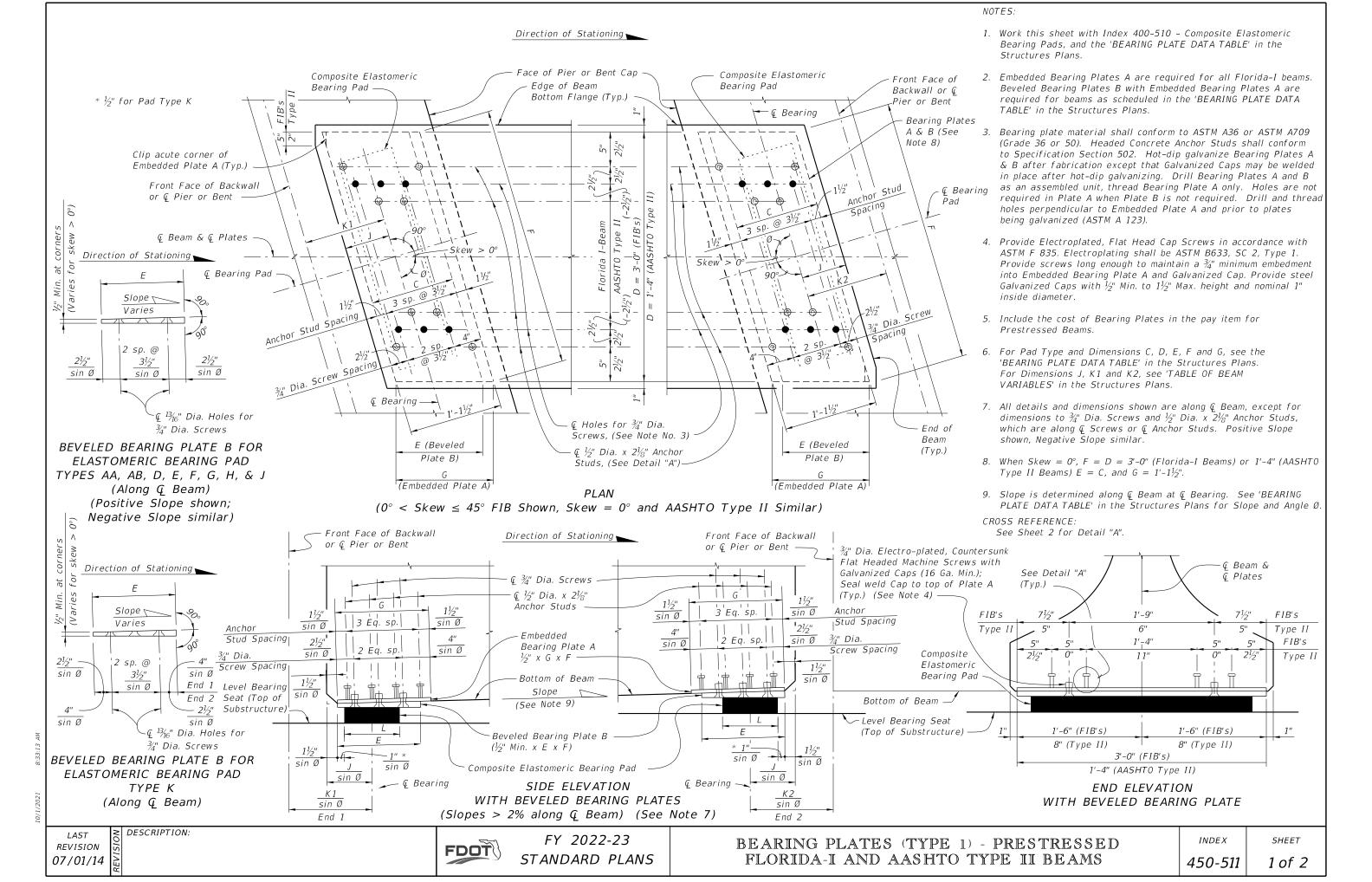
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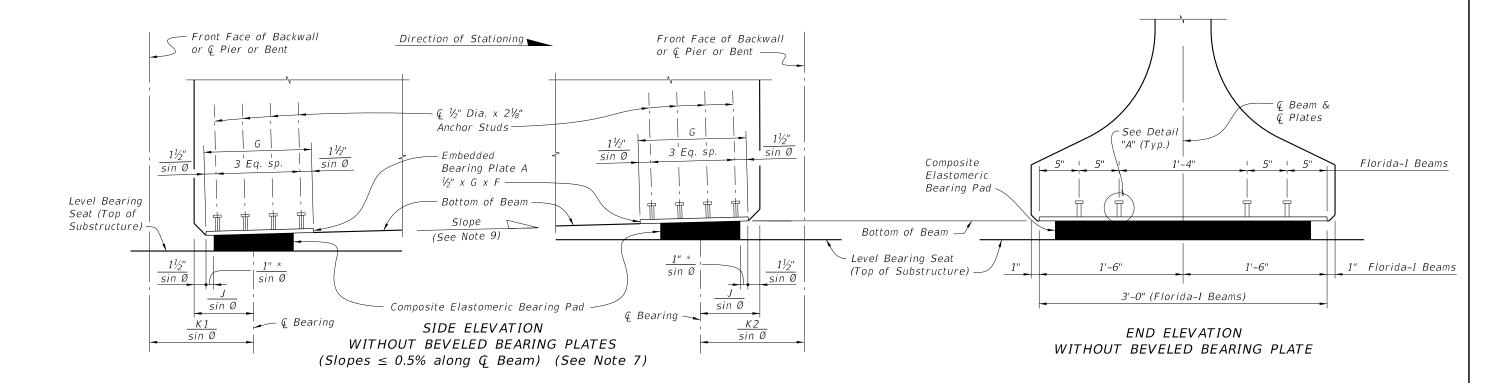




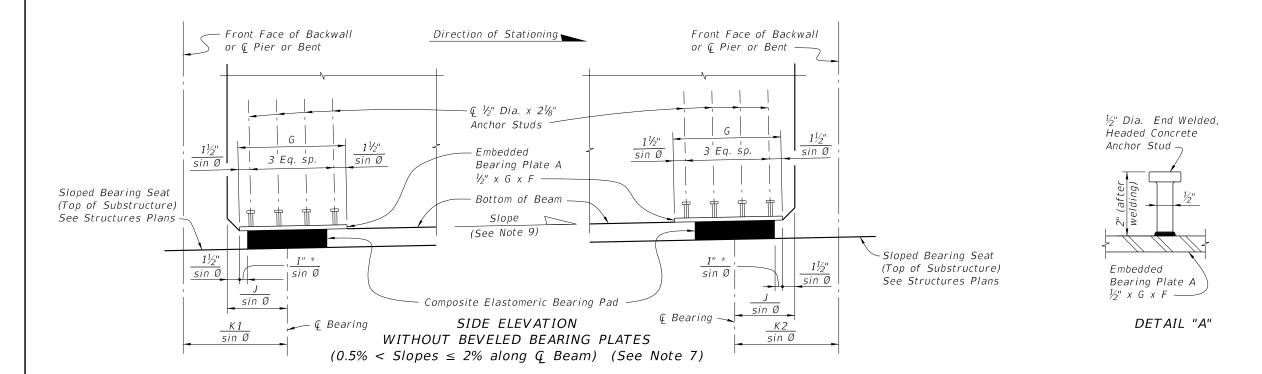








\* ½" Pad Type K

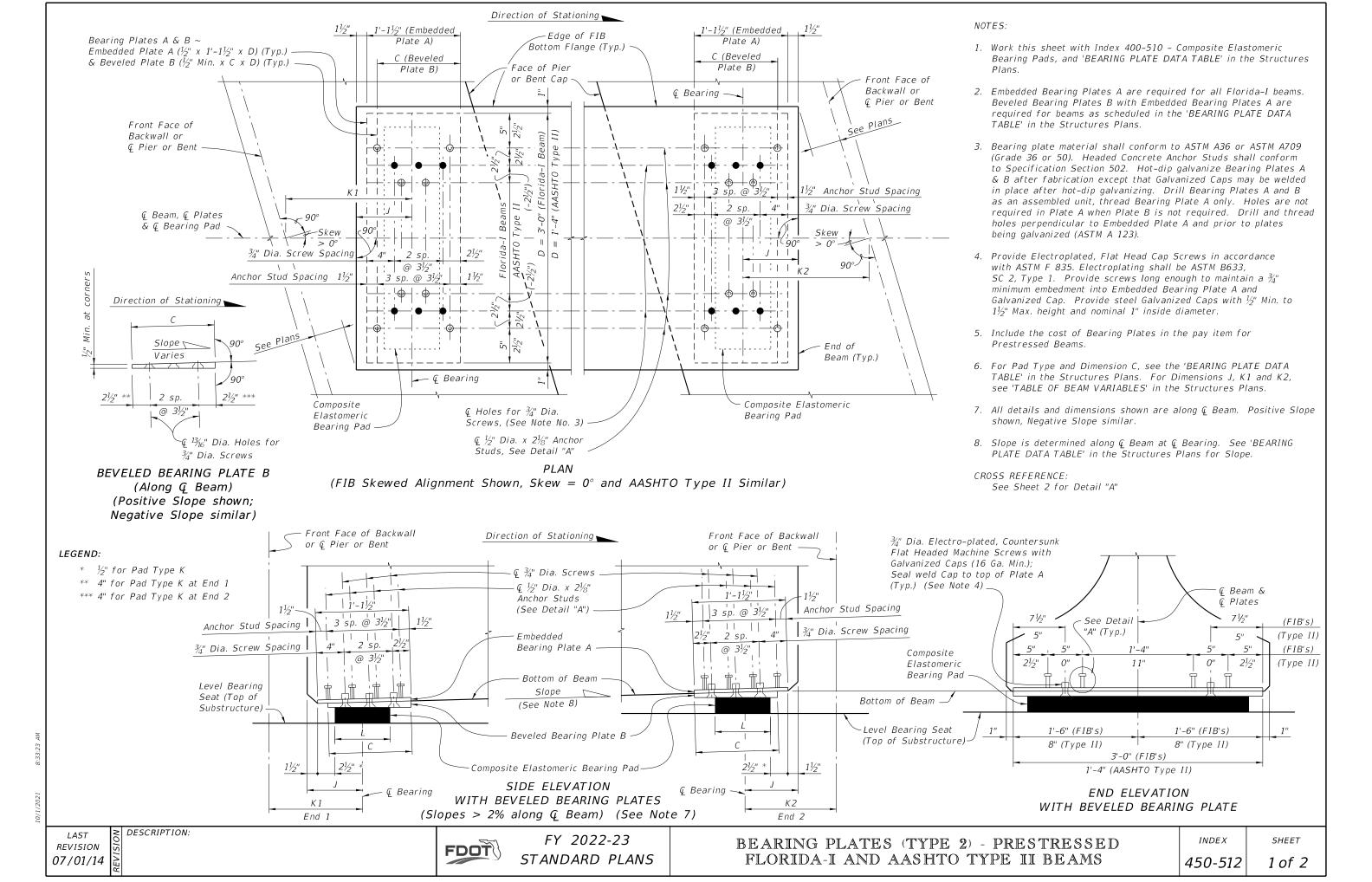


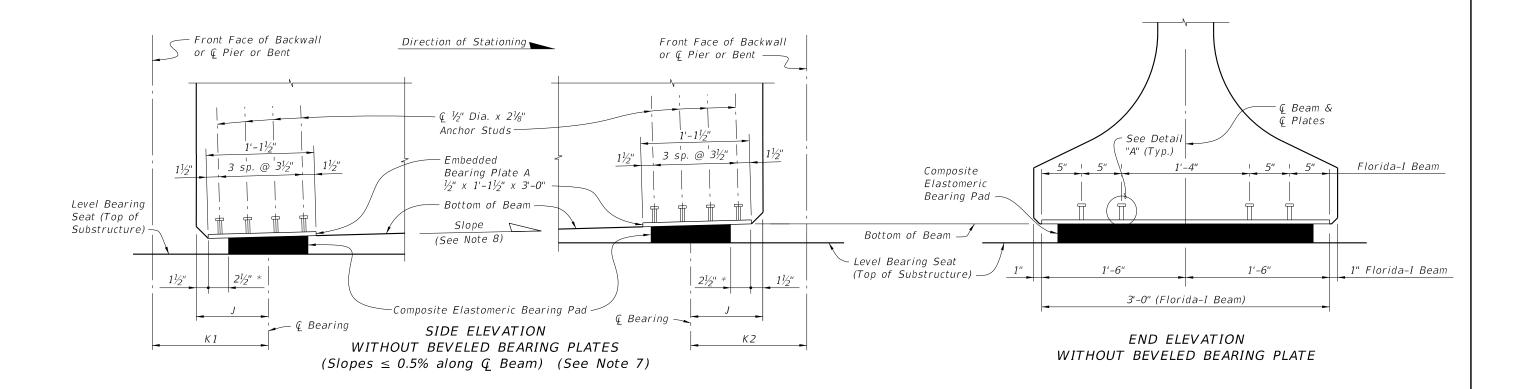
CROSS REFERENCE: See Sheet 1 for Notes.

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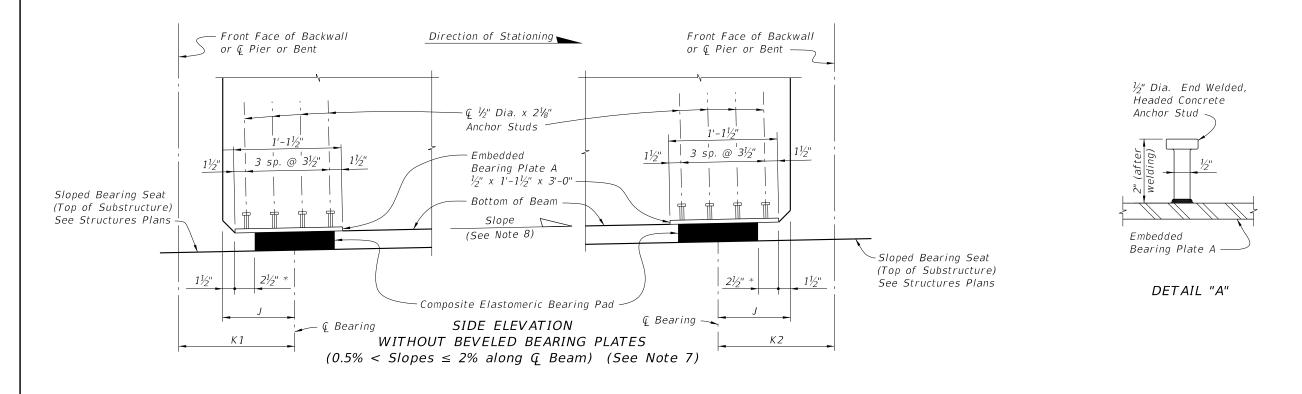
**REVISION** 11/01/20

FDOT





\* ½" for Pad Type K



CROSS REFERENCE: See Sheet 1 for Notes.

**REVISION** 07/01/14

DESCRIPTION:

450-512 2 of 2

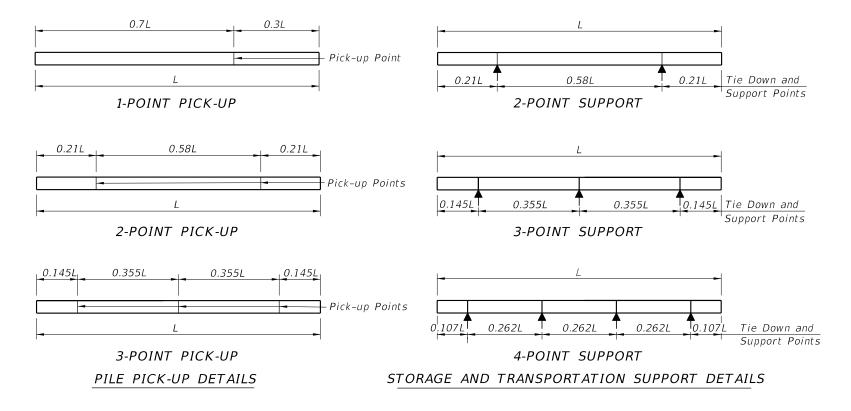
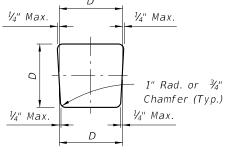
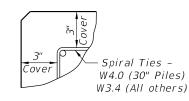


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = .	Square	e Pile	Size	(inches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum	48	52	59	68	87	2, 3, or 4 point	1 Point				
Pile Length (Feet)	69	75	85	98	124	2, 3, or 4 point	2 Point				
	99	107	121	140	178	3 or 4 point	3 Point				



TYPICAL PILE SHAPE FOR MOLD FORMS



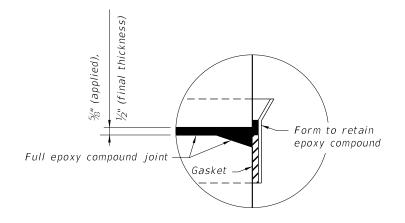
DETAIL SHOWING TYPICAL COVER

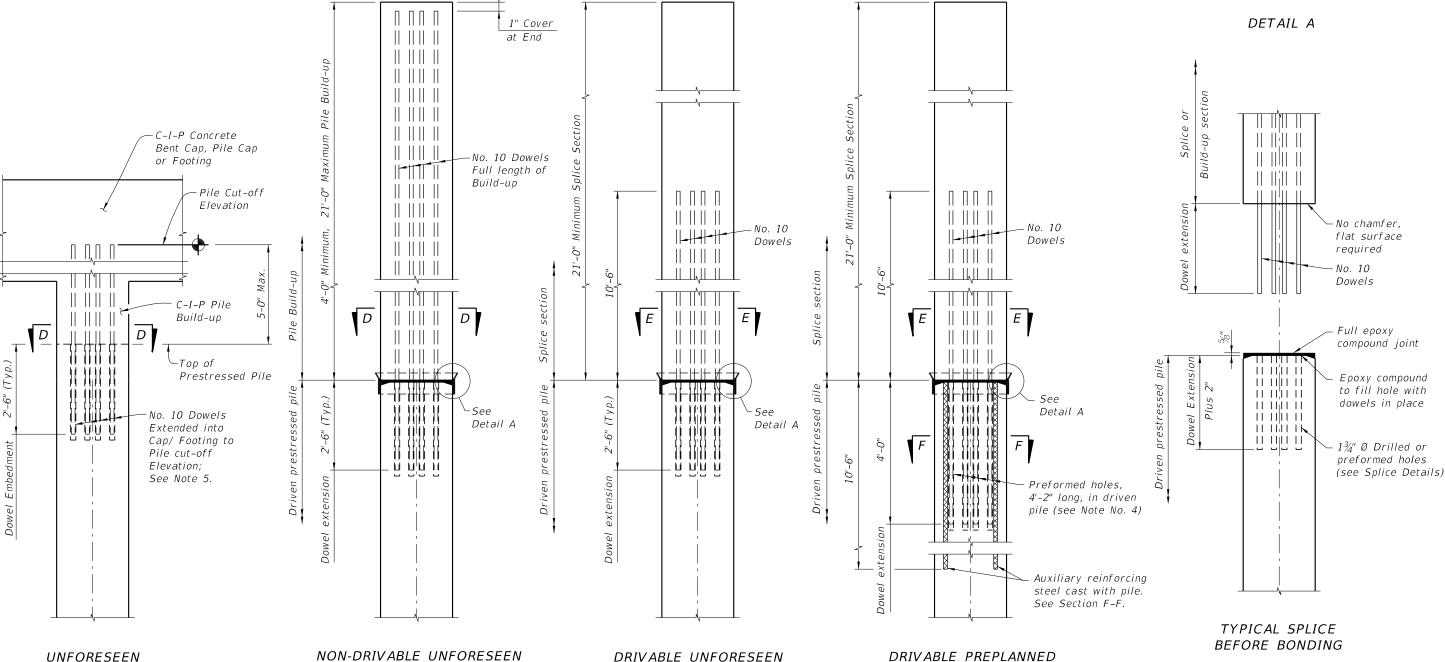
#### PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in the Structures Plans.
- 2. Concrete:
  - Piles: Class V (Special), except use Class VI for High Moment Capacity Pile (Index 455-031).
  - B. High Capacity Splice Collar: Class V (Special).
  - C. See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required.
- 3. Concrete strength at time of prestress transfer:
  - A. Piles: 4,000 psi minimum.
  - B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
  - A. Bars: Meet the requirements of Specification Section 415.
  - 3. Prestressing Strands: Meet the requirements of Specification Section 933.
  - Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
  - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
- B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 962. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

8:33:37 AM

- 1. For Sections D-D, E-E, & F-F see Index 455-012 thru 455-030 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- In cases where pile splices are desired due to length limitations in shipping and/or handling, the "Drivable Preplanned Prestressed Precast Splice Detail" shall be used. Mechanical Pile Splices contained on the Approved Products List (APL) may also be used.
- When preformed dowel holes are utilized, the 1" spiral tie pitch shall be continued to 4'-0" below the head of the pile, See Index 455-018, 455-020 & 455-024. Preformed holes shall utilize either removable preforming material or stay-in-place corrugated galvanized steel ducts. Stay-in-place ducts shall be fabricated from galvanized sheet steel meeting the requirements of ASTM A653, Coating Designation G90, 26 gauge. Ducts shall be 2" diameter with a minimum corrugation (rib) height of 0.12 in. Ducts shall be fabricated with either welded or interlocked seams. Galvanizing of welded seams will not be required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 10 Dowels into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.





PRESTRESSED PRECAST

PILE SPLICE DETAIL

DESCRIPTION: REVISION 07/01/14

REINFORCED C-I-P

PILE BUILD-UP DETAIL



REINFORCED PRECAST

PILE BUILD-UP DETAIL

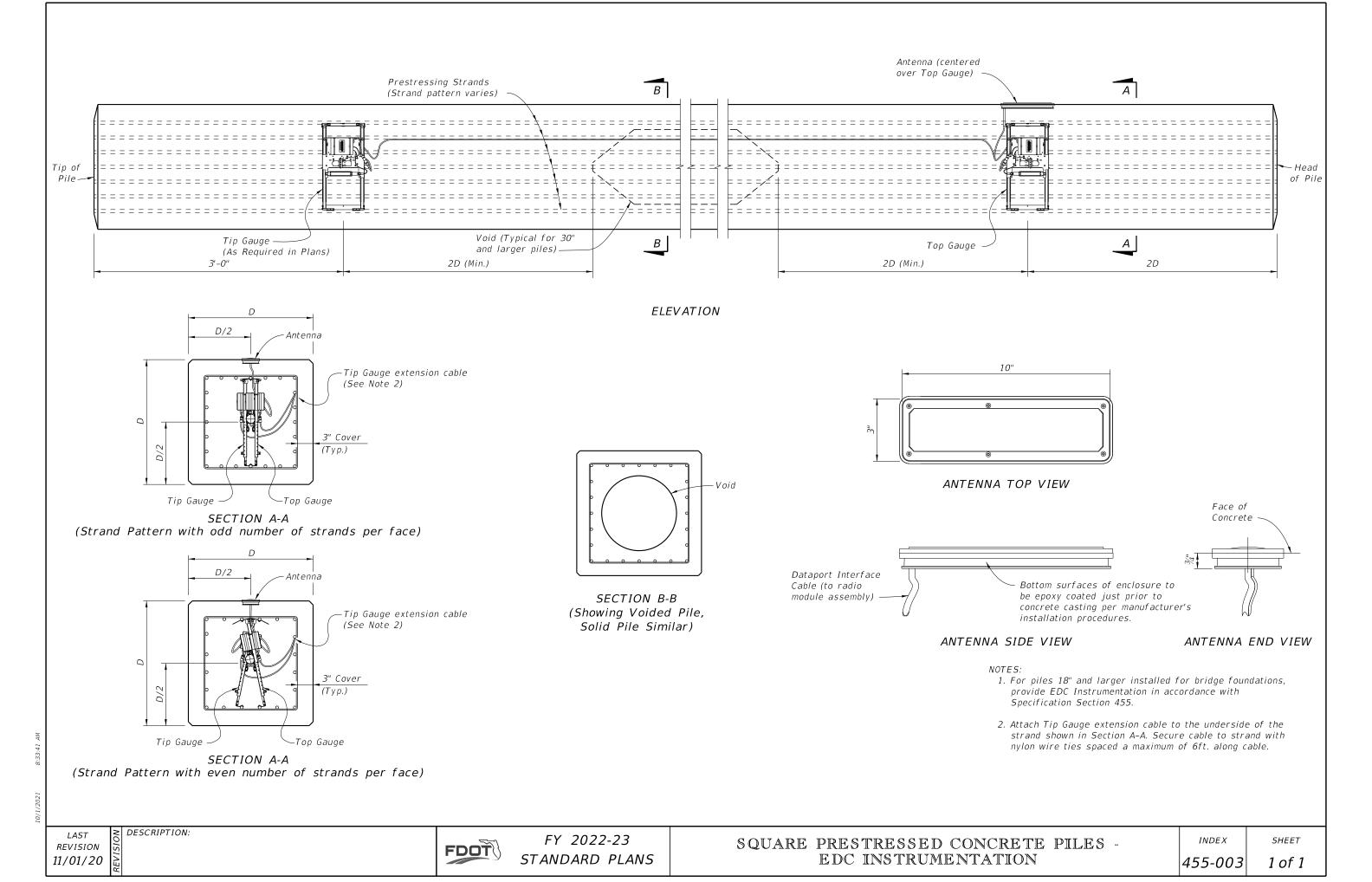
FY 2022-23 STANDARD PLANS

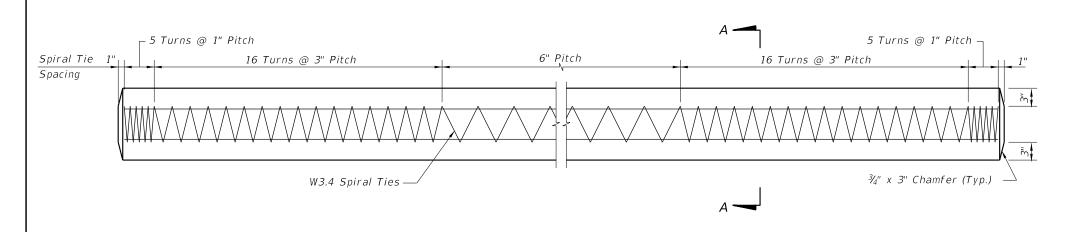
INDEX *455-002* 

SHEET 1 of 1

PRESTRESSED PRECAST

PILE SPLICE DETAIL





# ALTERNATE STRAND PATTERNS

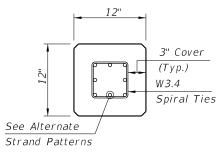
4 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

 $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 25 kips

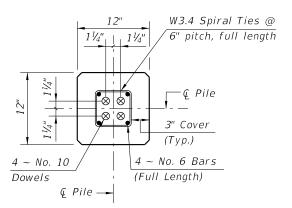
 $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 24 kips

 $8 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 23 kips

12 ~ ¾" Ø, Grade 270 LRS, at 16 kips

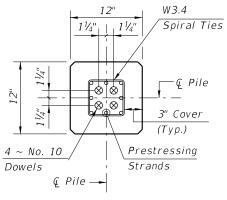


SECTION A-A



#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



#### SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

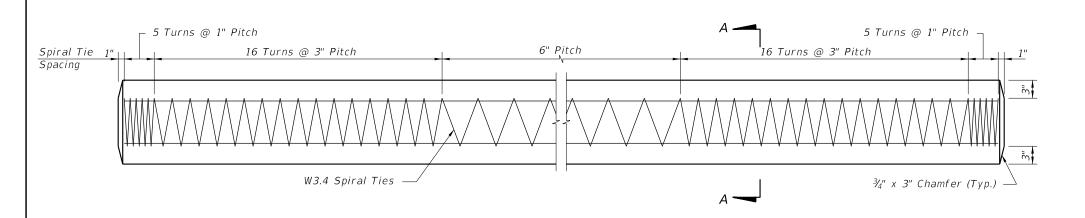
# PILE SPLICE REINFORCEMENT DETAILS

# NOTES:

- 1. Work this Index with Index 450-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

  Place one strand at each corner and place the remaining

strands equally spaced between the corner strands.
The total strand pattern shall be concentric with the nominal concrete section of the pile.

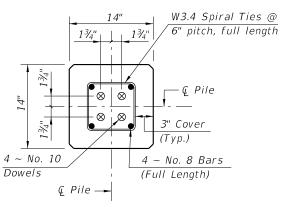


# 3" Cover (Typ.) W3.4 Spiral Ties See Alternate Strand Patterns

#### SECTION A-A

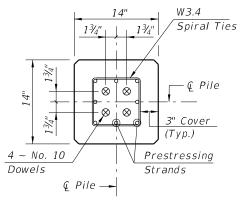
# ALTERNATE STRAND PATTERNS

- 8 ~ 0.6" Ø, Grade 270 LRS, at 33 kips
- $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 31 kips
- $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips
- $12 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips
- 16 ~ ¾" Ø, Grade 270 LRS, at 16 kips



#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



#### SECTION E-E

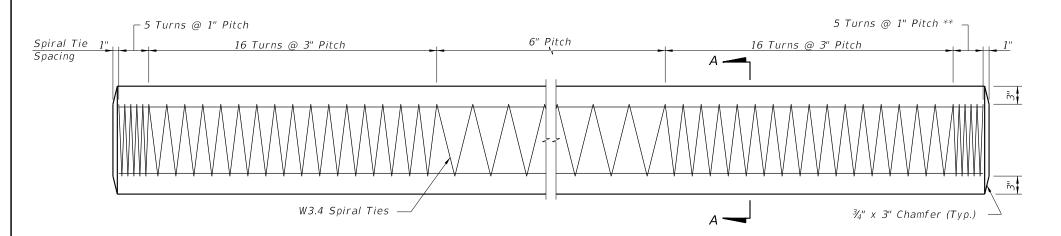
(See Drivable Unforeseen Prestressed Precast Splice Detail)

# PILE SPLICE REINFORCEMENT DETAILS

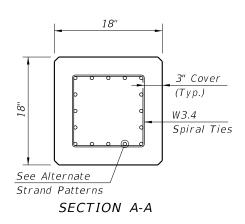
#### NOTES:

- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



\*\* See Note 4 on Index 455-002



# ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips

 $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

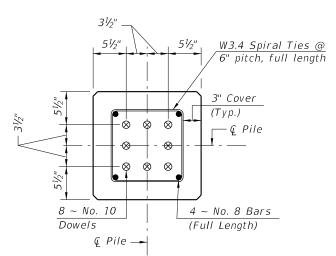
 $16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips

 $20 \sim \frac{7}{16}$  Ø, Grade 270 LRS, at 21 kips

 $24 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 17 kips

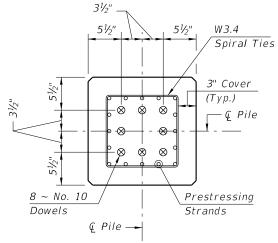
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



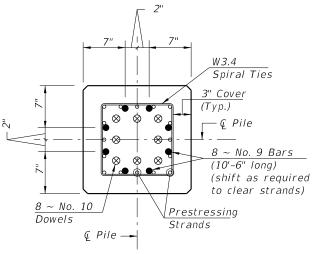
#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



#### SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



#### SECTION F-F

(See Drivable Preplanned Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

**REVISION** 01/01/12

DESCRIPTION:

FDOT

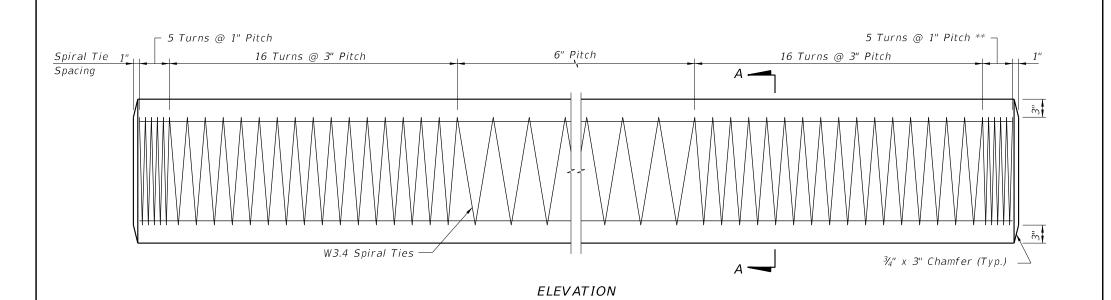
FY 2022-23 STANDARD PLANS

18" SQUARE PRESTRESSED CONCRETE PILE

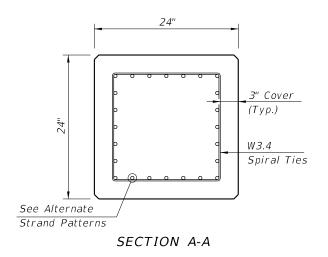
INDEX

SHEET

455-018 1 of 1



\*\* See Note 4 on Index 455-002



# ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

 $20 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

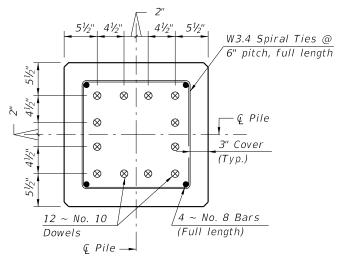
24 ~ ½" Ø, Grade 270 LRS, at 31 kips

#### NOTES:

- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

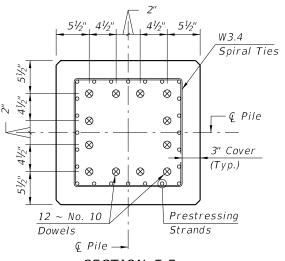
Place one strand at each corner and place the remaining strands equally spaced between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.



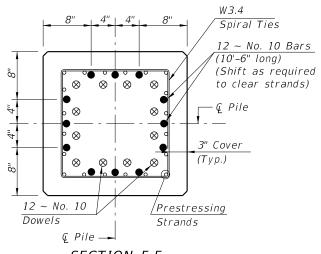
# SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



# SECTION E-E

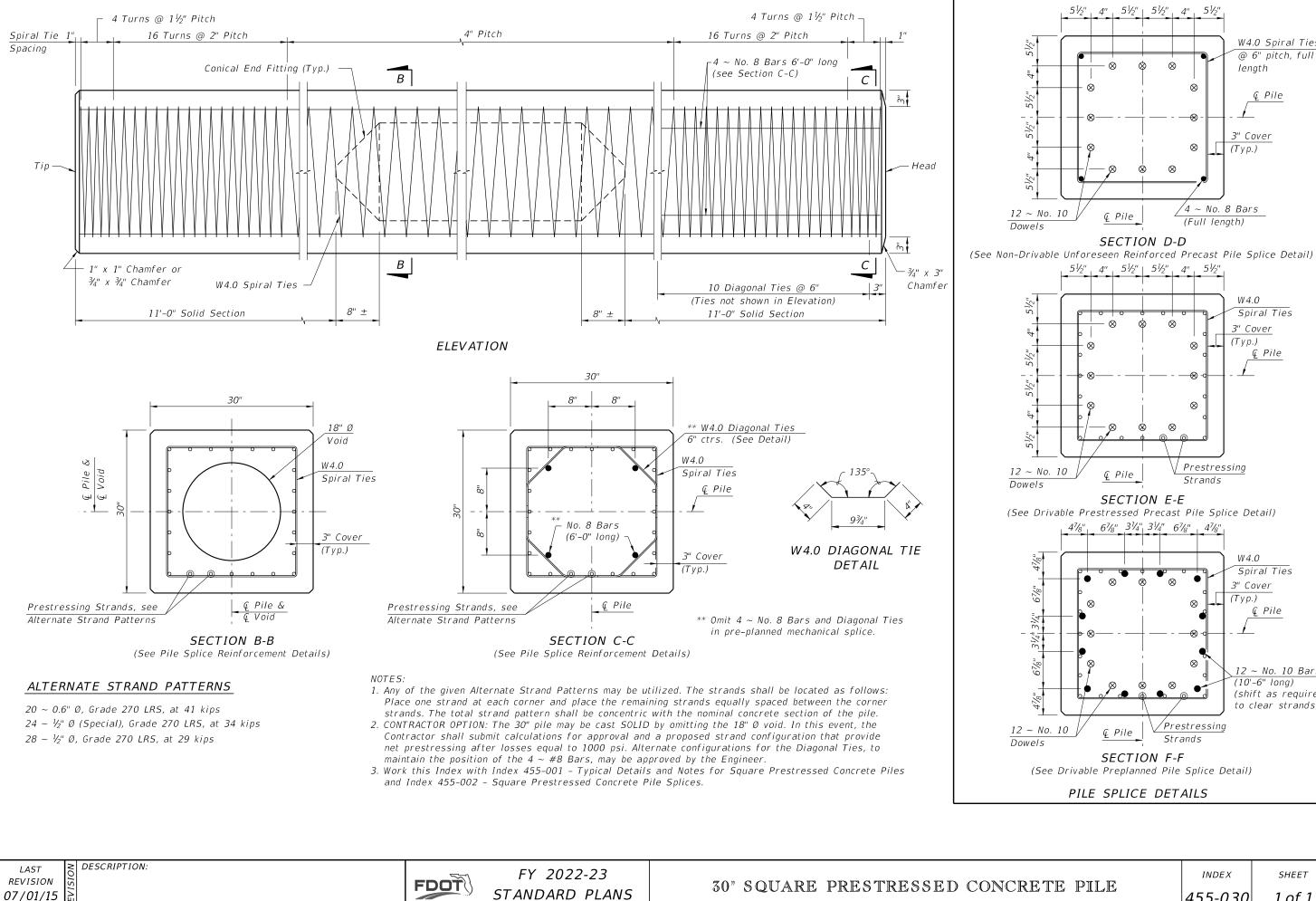
(See Drivable Prestressed Precast Pile Splice Detail)



#### SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

10/1/2021



SHEET

W4.0 Spiral Ties

@ 6" pitch, full

€ Pile

length

3" Cover (Typ.)

W4.0

3" Cover (Typ.)

W4.0

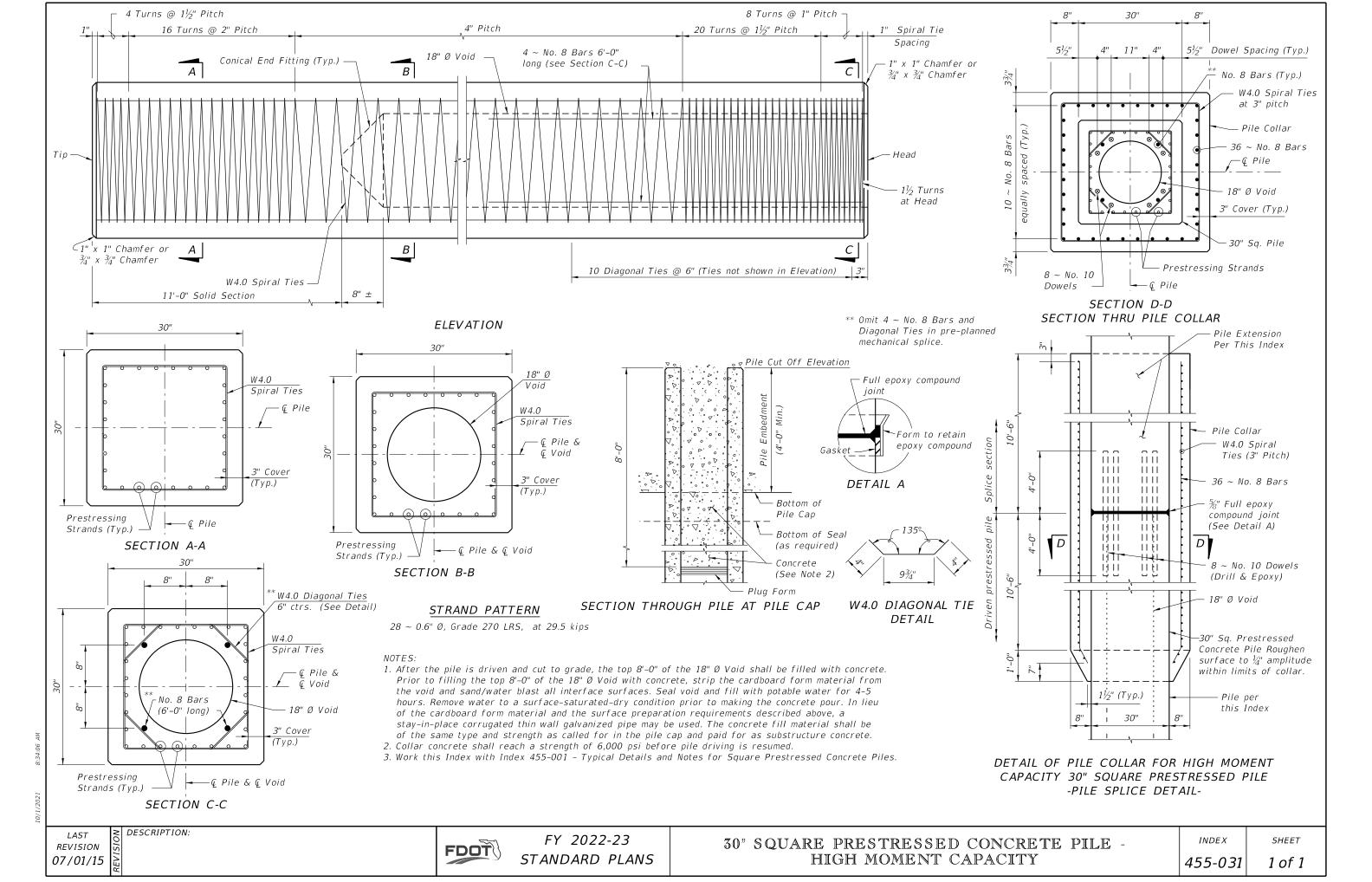
Spiral Ties 3" Cover (Typ.)

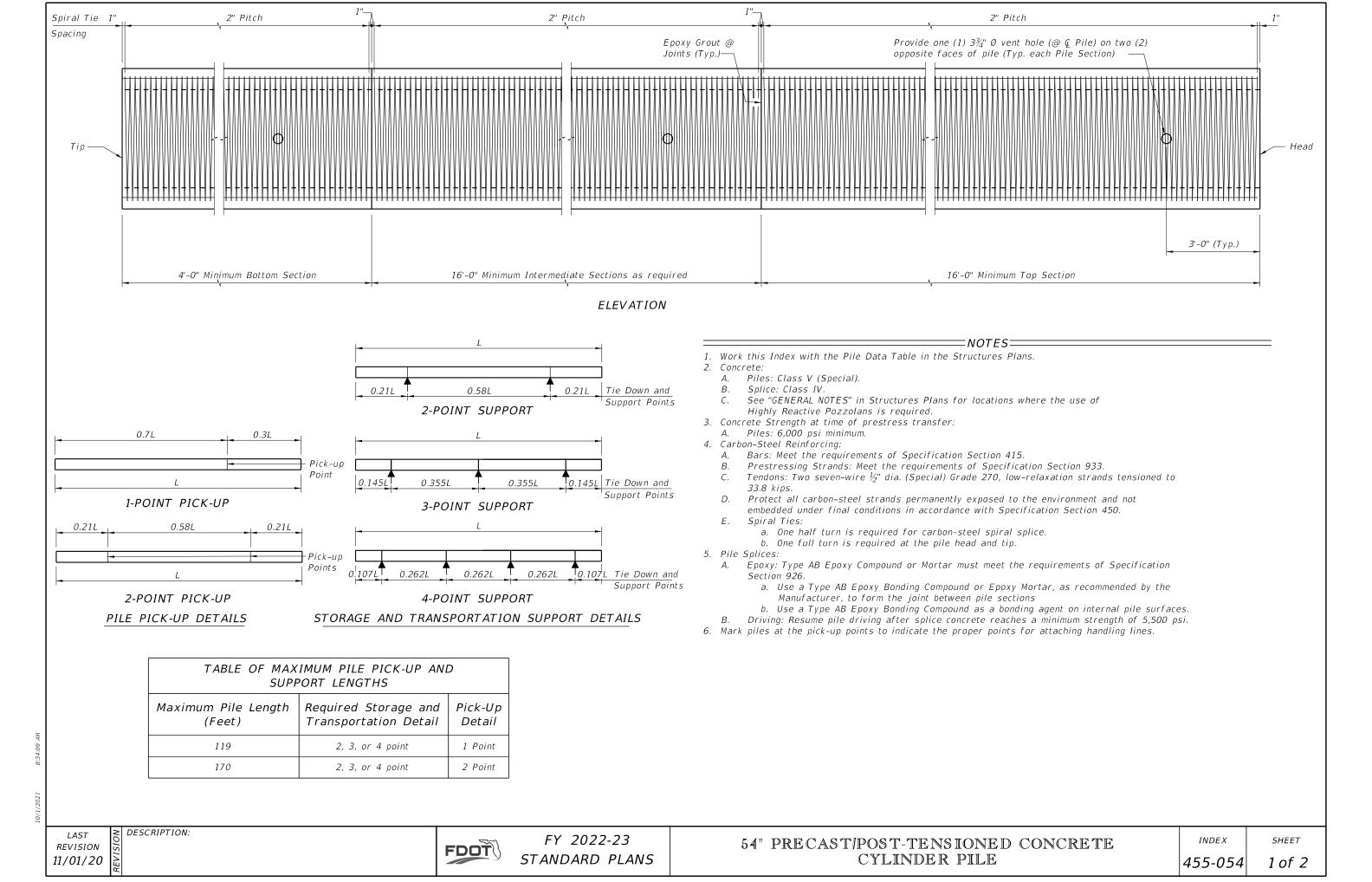
12 ~ No. 10 Bars (10'-6" long)

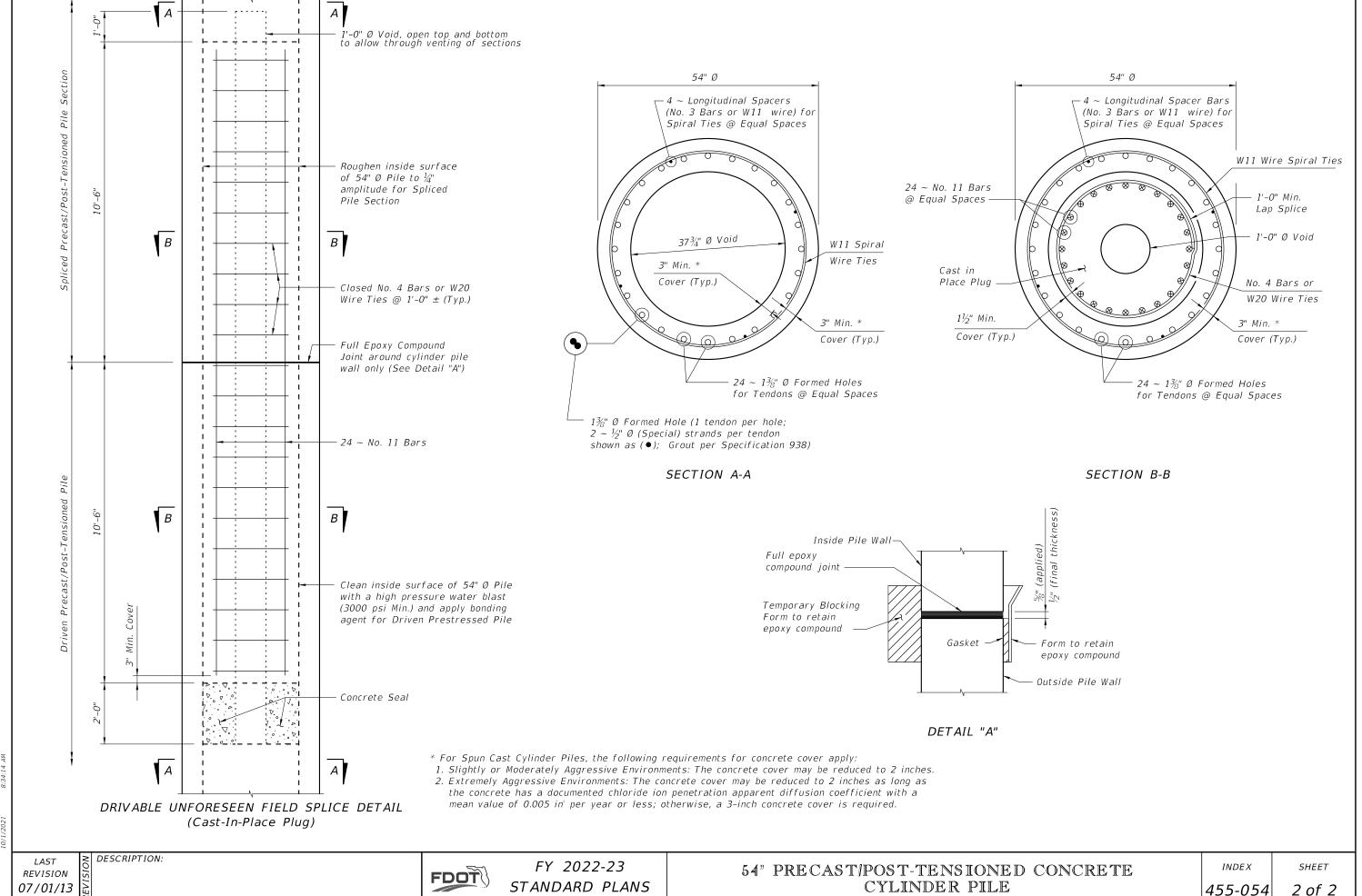
(shift as required

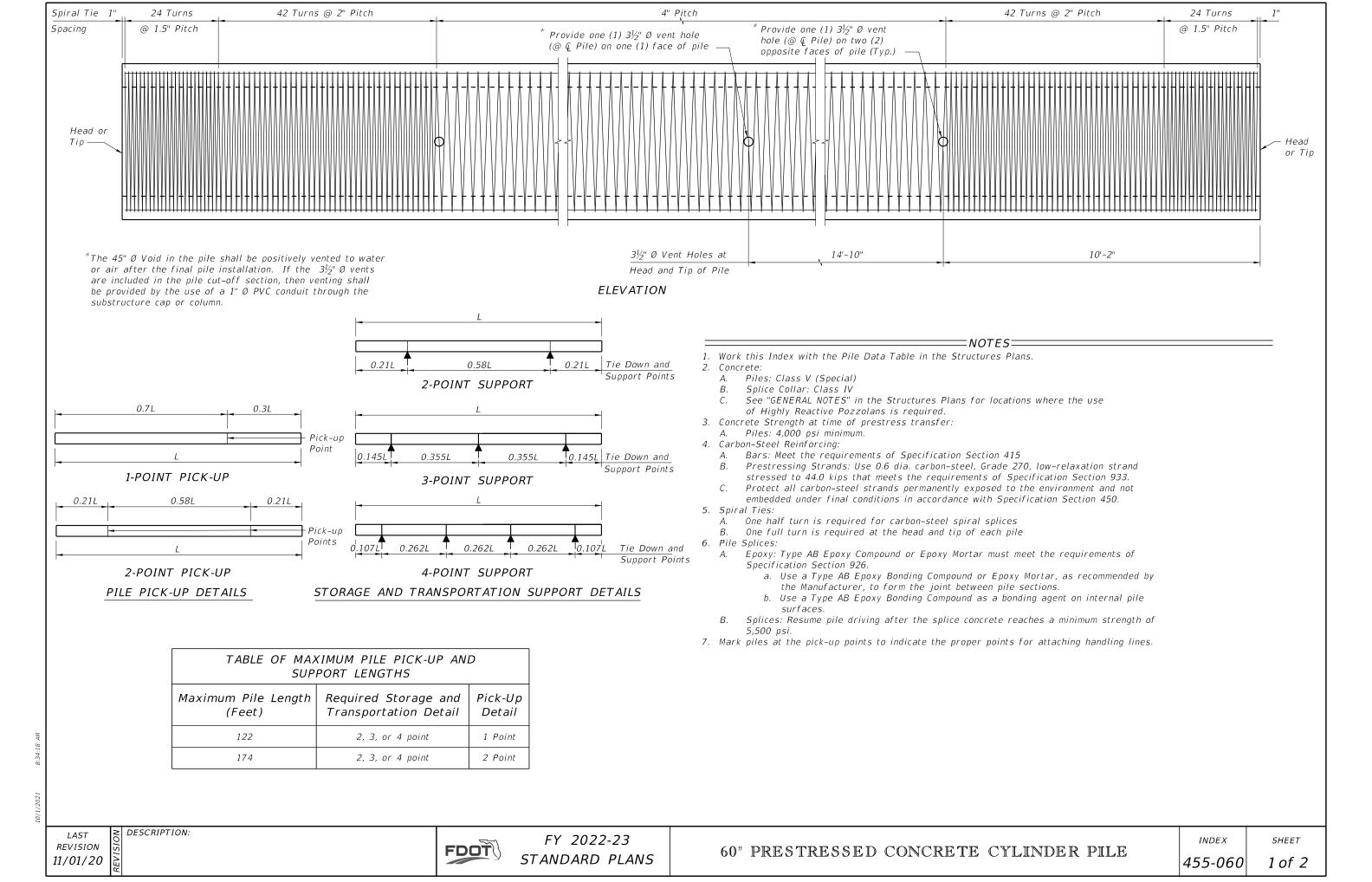
to clear strands)

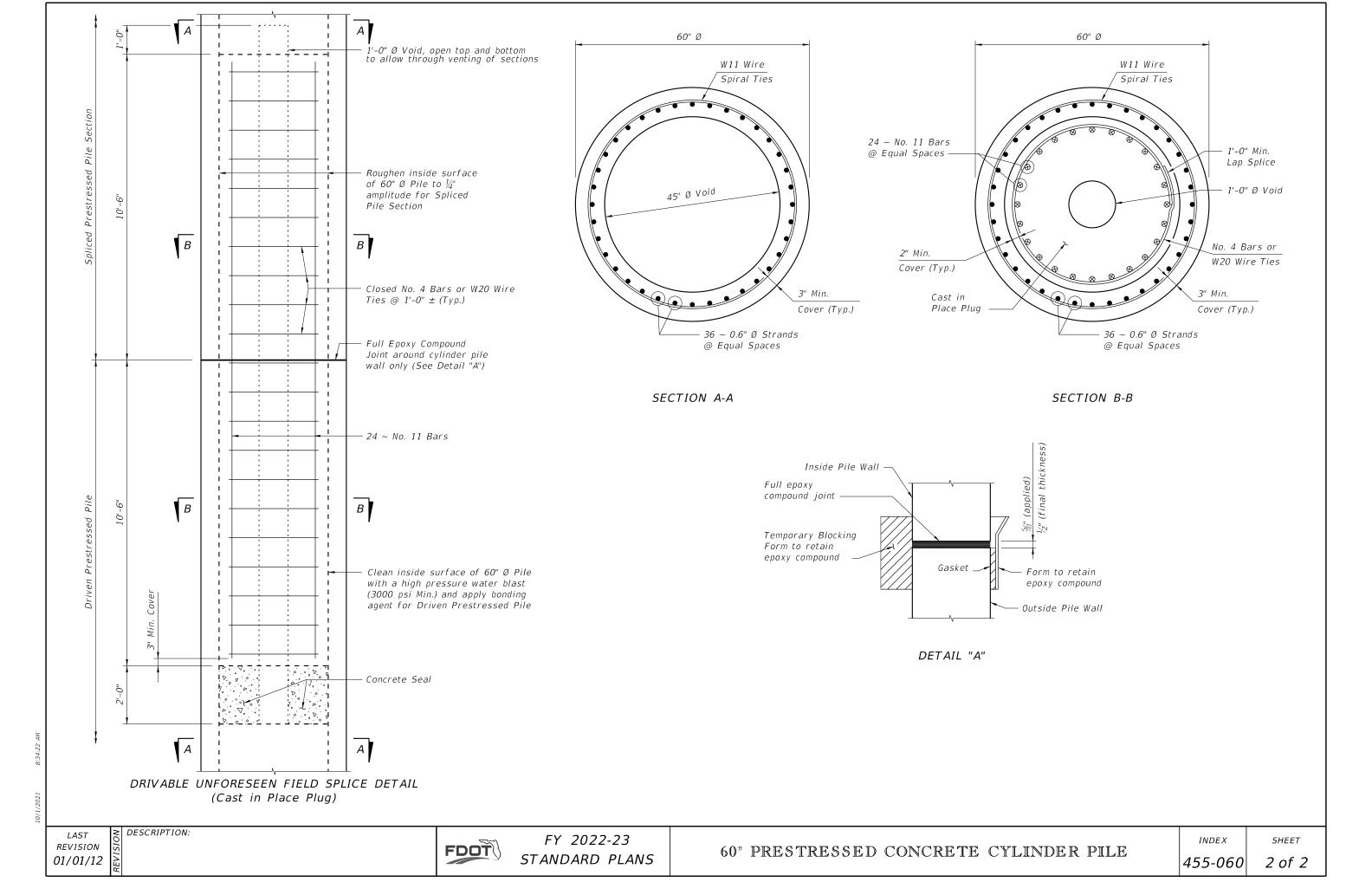
Spiral Ties











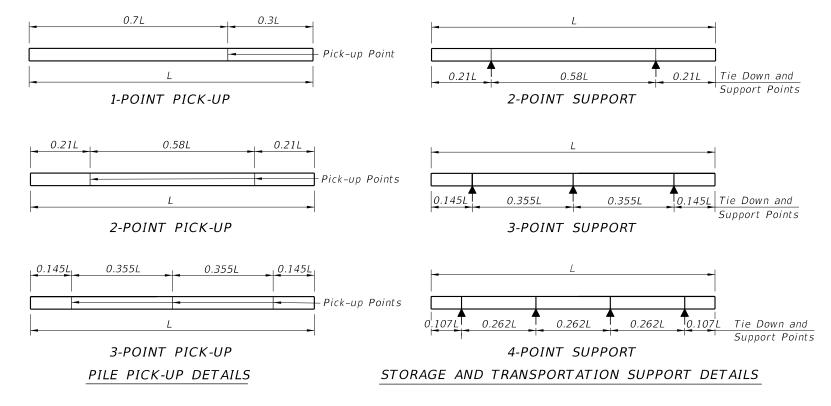
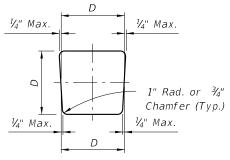
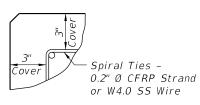


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS												
	D = S	Square	Pile S	ize (ir	ches)	Required Storage and Transportation Detail	Pick-Up Detail					
	12	14	18	24	30							
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point					
	69	75	85	98	124	2, 3, or 4 point	2 Point					
	99	107	121	140	178	3 or 4 point	3 Point					



TYPICAL PILE SHAPE FOR MOLD FORMS

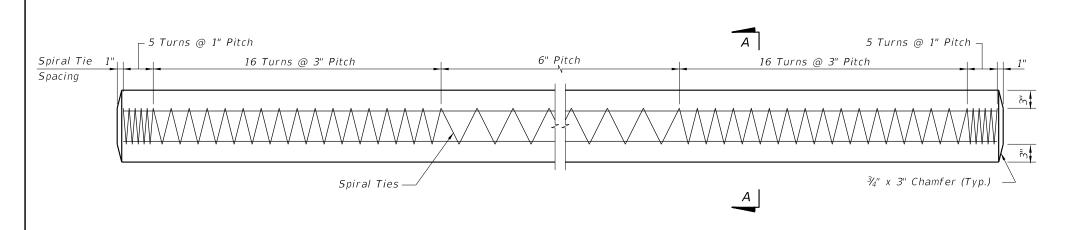


DETAIL SHOWING TYPICAL COVER

#### PRESTRESSED CONCRETE PILE NOTES:

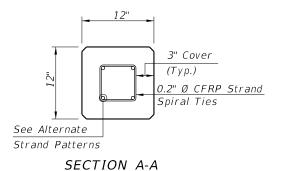
- Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-102), the Prestressed Concrete Pile Standards (Index 455-112, 455-114, 455-118, 455-124, 455-130, and the Pile Data Table in the Structures Plans.
- 2. Concrete:
  - A. Piles: Class V (Special)
  - . See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required for options using stainless steel strand and reinforcing.
- 3. Concrete strength at time of prestress transfer:
  - A. Piles: 4,000 psi minimum.
- 4. Reinforcing:
  - A. Bars:
    - a. Stainless Steel: Meet the requirements of Specification Section 931 for Type 304, Grade 75.
    - b. Carbon FRP: Meet the requirements of Specification Section 932.
  - B. Prestressing Strands:
    - a. Stainless Steel: Seven-wire HSSS, Grade 240 strand, meeting the requirements of Specification Section 933.
    - b. Carbon FRP: Meet the requirements of Specification Section 933.
- 5. Spiral Ties:
  - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
  - B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

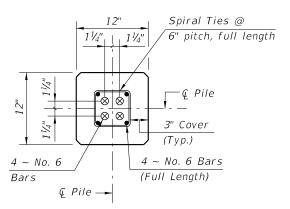




#### ALTERNATE STRAND PATTERNS

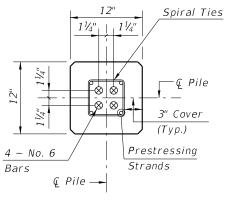
4 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips  $4 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 41 kips





#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



# SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

# CFRP PILE SPLICE REINFORCEMENT DETAILS

# NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized.

CFRP PRESTRESSED PILE DETAILS

**REVISION** 11/01/16

DESCRIPTION:

FDOT

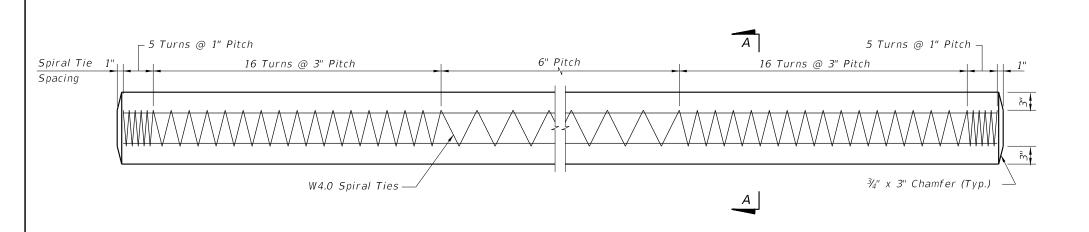
FY 2022-23 STANDARD PLANS

12" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

SHEET

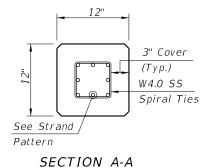
455-112 1 of 2

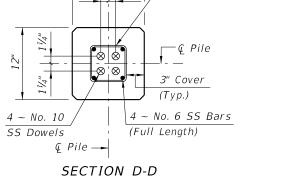


**ELEVATION** 

# STRAND PATTERN

 $8 \sim \frac{1}{2}$ " Ø, HSSS at 24 kips 8 ~ 0.6" Ø, HSSS at 26 kips

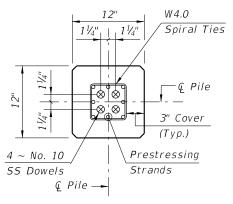




W4.0 Spiral Ties @ 6" pitch, full length

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)

11/4" | 11/4"



# SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

# SS PILE SPLICE REINFORCEMENT DETAILS

# NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

**REVISION** 11/01/20

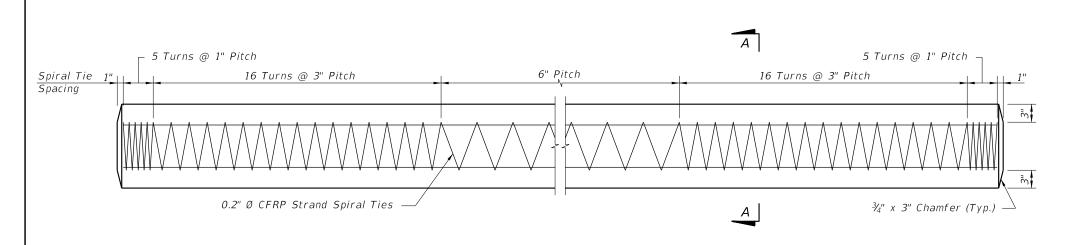
DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS 12" SQUARE CFRP & SS PRESTRESSED

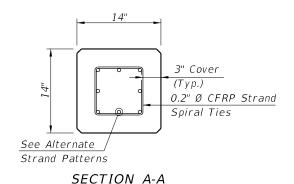
INDEX 455-112

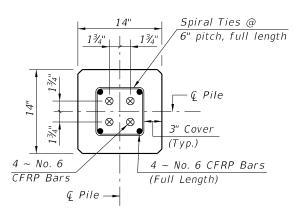
SHEET 2 of 2



# ALTERNATE STRAND PATTERNS

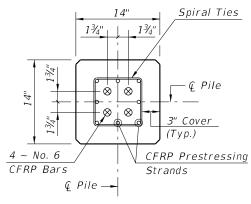
8 ~ 0.6" Ø, CFRP 7-Strand, at 31.5 kips  $8 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 30.5 kips





#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



#### SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

# CFRP PILE SPLICE REINFORCEMENT DETAILS

#### NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and equally space the remaining strands between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.

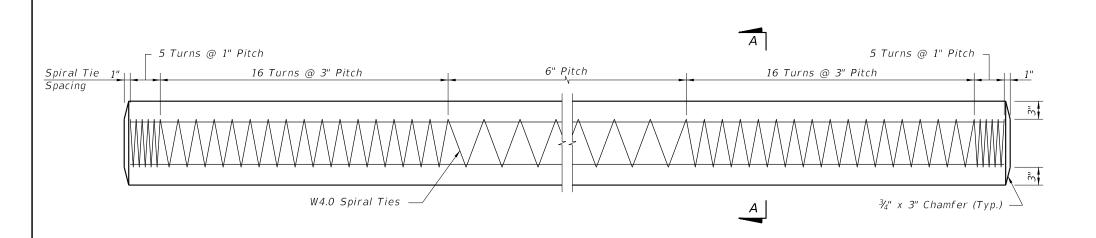
CFRP PRESTRESSED PILE DETAILS

**REVISION** 11/01/16

DESCRIPTION:

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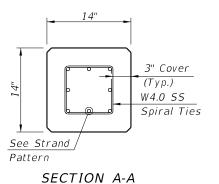
455-114 1 of 2

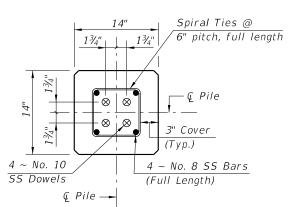


**ELEVATION** 

# STRAND PATTERN

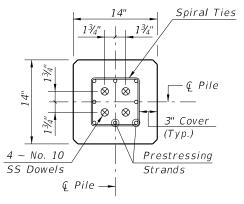
 $12 \sim \frac{1}{2}$ " Ø, HSSS at 23 kips  $8 \sim 0.6$ " Ø, HSSS at 35 kips





#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



# SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

# SS PILE SPLICE REINFORCEMENT DETAILS

#### NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

LAST REVISION 11/01/20

DESCRIPTION:

FDOT

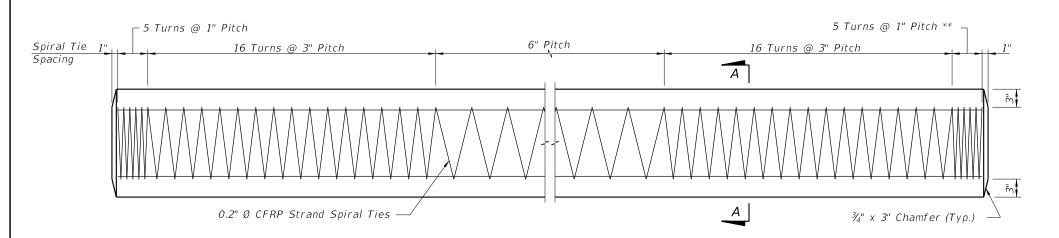
FY 2022-23 STANDARD PLANS

14" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

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*SHEET* 2 of 2

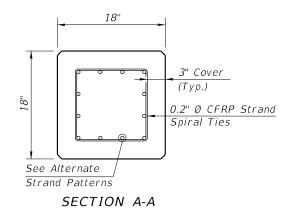
455-114



\*\* See Note 4 on Index 455-102

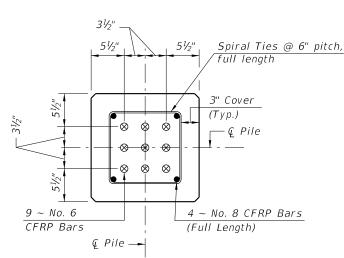
#### ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, CFRP 7-Strand, at 34 kips  $12 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 33 kips



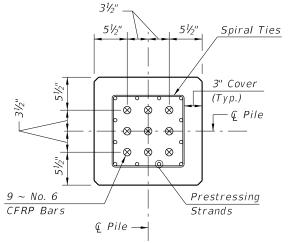
#### NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



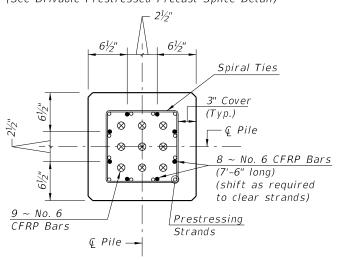
#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



# SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



#### SECTION F-F

(See Drivable Preplanned Prestressed Precast Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

CFRP PRESTRESSED PILE DETAILS

**REVISION** 11/01/16

DESCRIPTION:

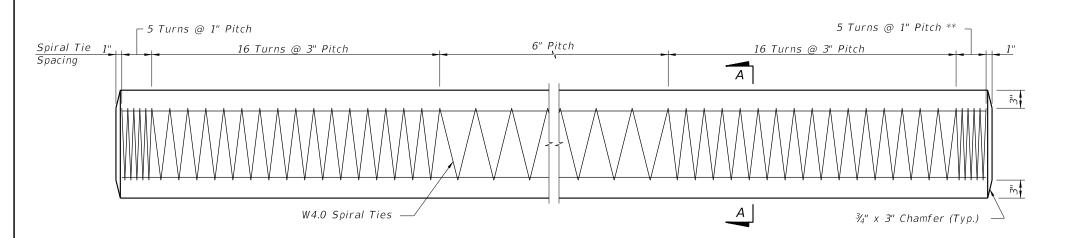
FDOT

FY 2022-23 STANDARD PLANS

18" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

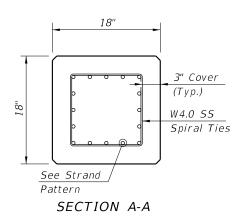
SHEET



\*\* See Note 4 on Index 455-102

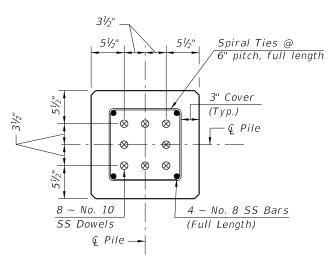
# STRAND PATTERN

 $16 \sim \frac{1}{2}$ " Ø, HSSS, at 26 kips 12 ~ 0.6" Ø, HSSS, at 35 kips



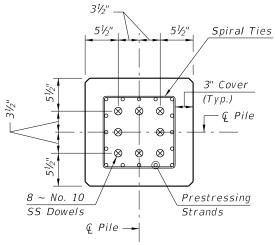
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



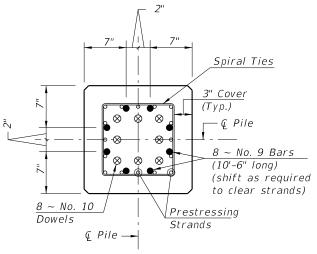
#### SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



#### SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



#### SECTION F-F

(See Drivable Preplanned Prestressed Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

**REVISION** 11/01/20

DESCRIPTION:

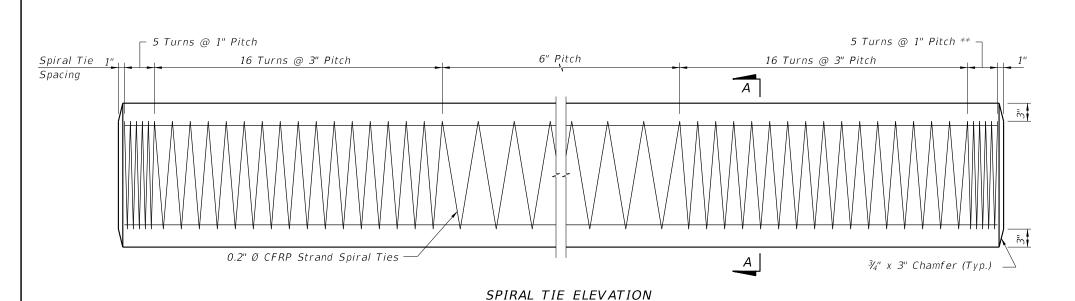
FDOT

FY 2022-23 STANDARD PLANS

18" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

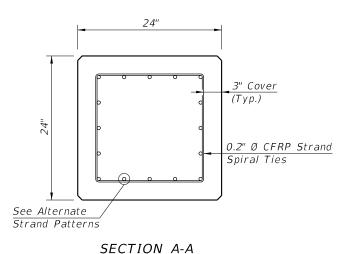
SHEET



\*\* See Note 4 on Index 455-102

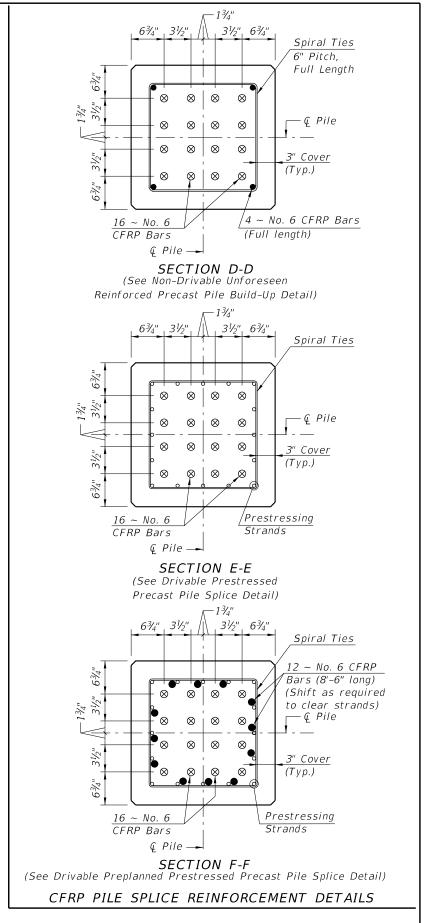
# ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips  $20 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 35 kips



- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.



CFRP PRESTRESSED PILE DETAILS

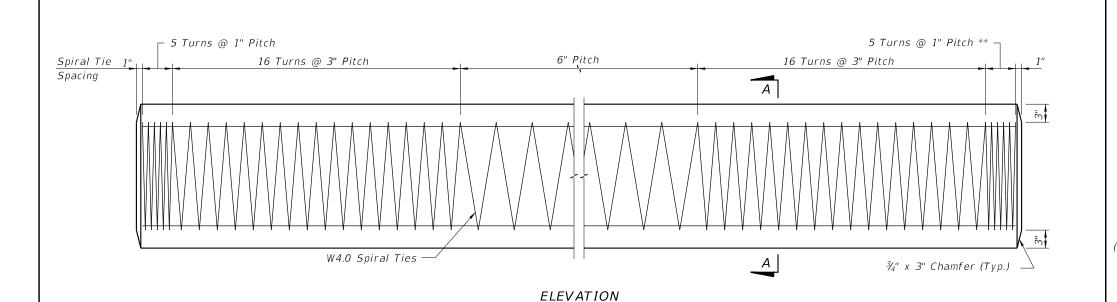
**REVISION** 11/01/20

DESCRIPTION:

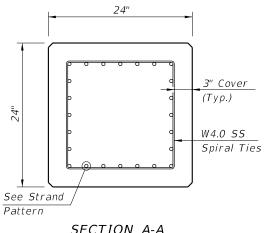
FDOT

FY 2022-23 STANDARD PLANS

24" SQUARE CFRP & SS PRESTRESSED



\*\* See Note 4 on Index 455-102



# STRAND PATTERN

 $28 \sim \frac{1}{2}$ " Ø, HSSS at 26 kips 20 ~ 0.6" Ø, HSSS at 35 kips

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index
- The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands.

SS PRESTRESSED PILE DETAILS

**REVISION** 11/01/20

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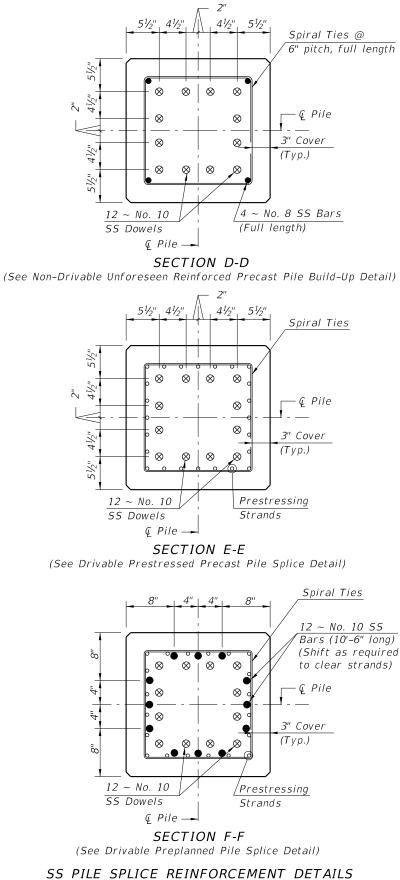
FDOT

FY 2022-23 STANDARD PLANS 24" SQUARE CFRP & SS PRESTRESSED

INDEX

SHEET 2 of 2

Pattern SECTION A-A

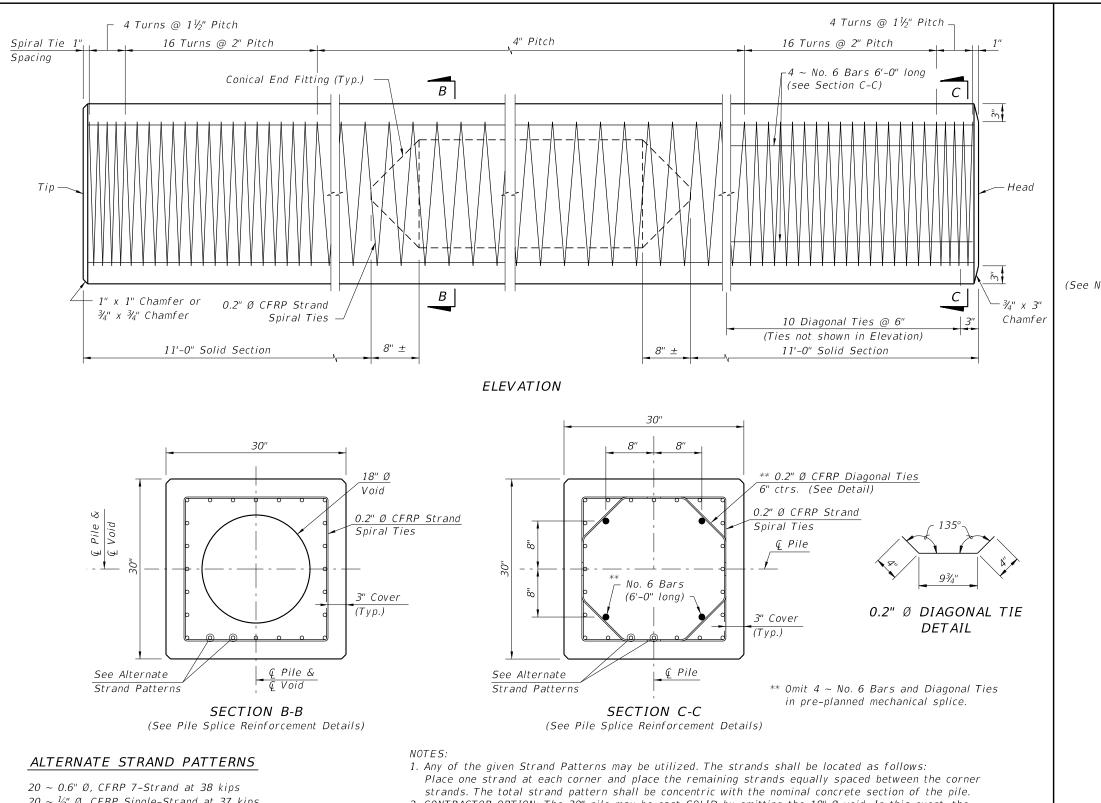


455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.

2. Any of the given Strand Patterns may be utilized.

The total strand pattern shall be concentric with the nominal

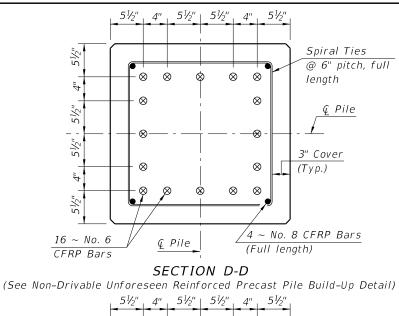
concrete section of the pile.

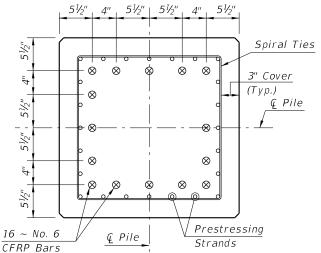


 $20 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand at 37 kips

DESCRIPTION:

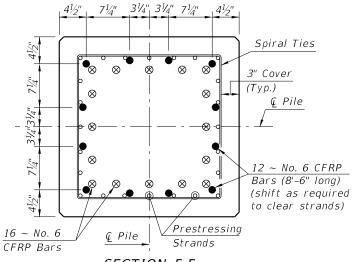
- 2. CONTRACTOR OPTION: The 30" pile may be cast SOLID by omitting the 18" Ø void. In this event, the Contractor shall submit calculations for approval and a proposed strand configuration that provide net prestressing after losses equal to 1000 psi. Alternate configurations for the Diagonal Ties, to maintain the position of the  $4\sim$  #6 Bars, may be approved by the Engineer.
- 3. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.





# SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



## SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE DETAILS

CFRP PRESTRESSED PILE DETAILS

**REVISION** 11/01/16

FDOT

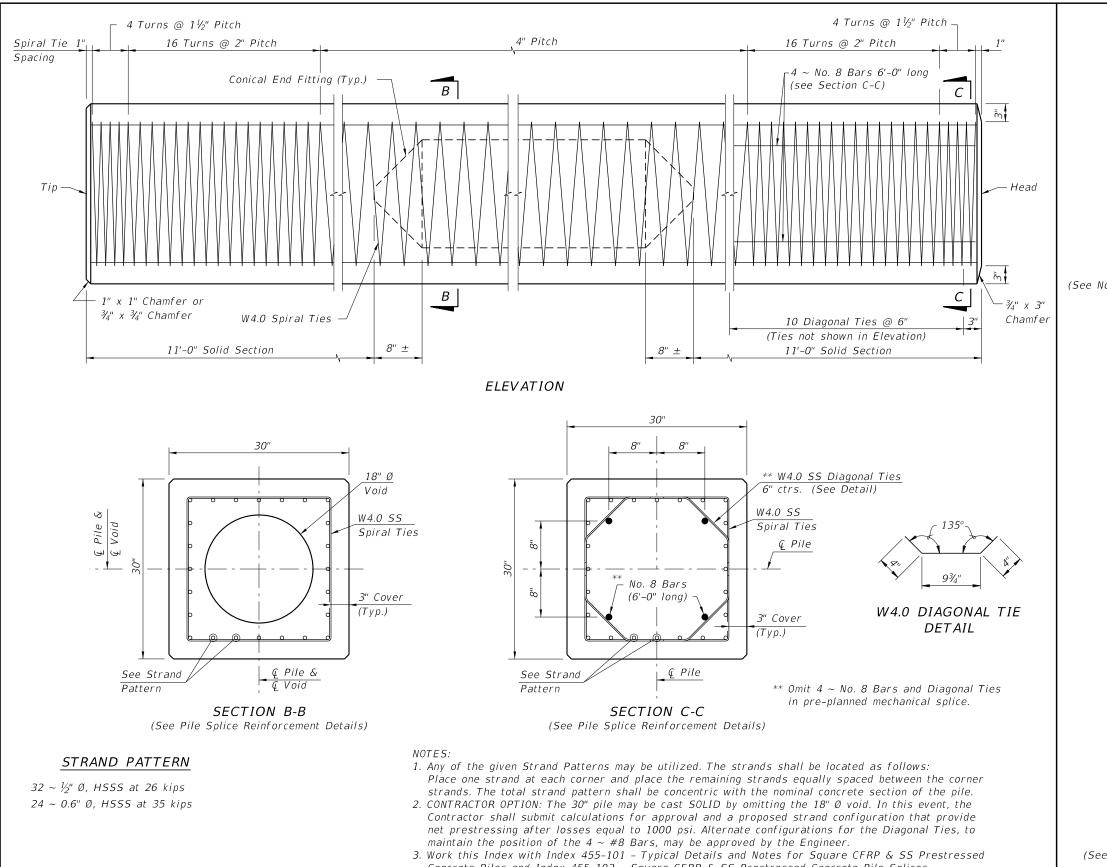
FY 2022-23 STANDARD PLANS

30" SQUARE CFRP & SS PRESTRESSED

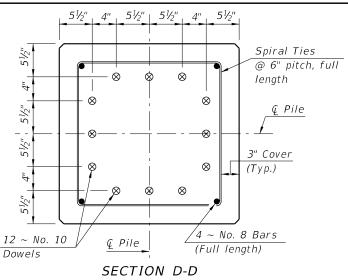
INDEX

SHEET 1 of 2

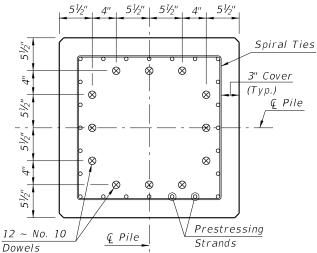
455-130



Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.

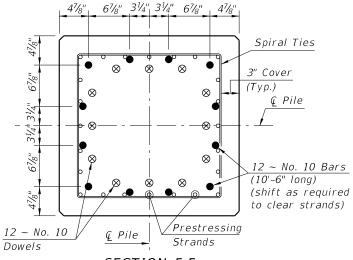


(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



# SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



## SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

SS PILE SPLICE DETAILS

SS PRESTRESSED PILE DETAILS

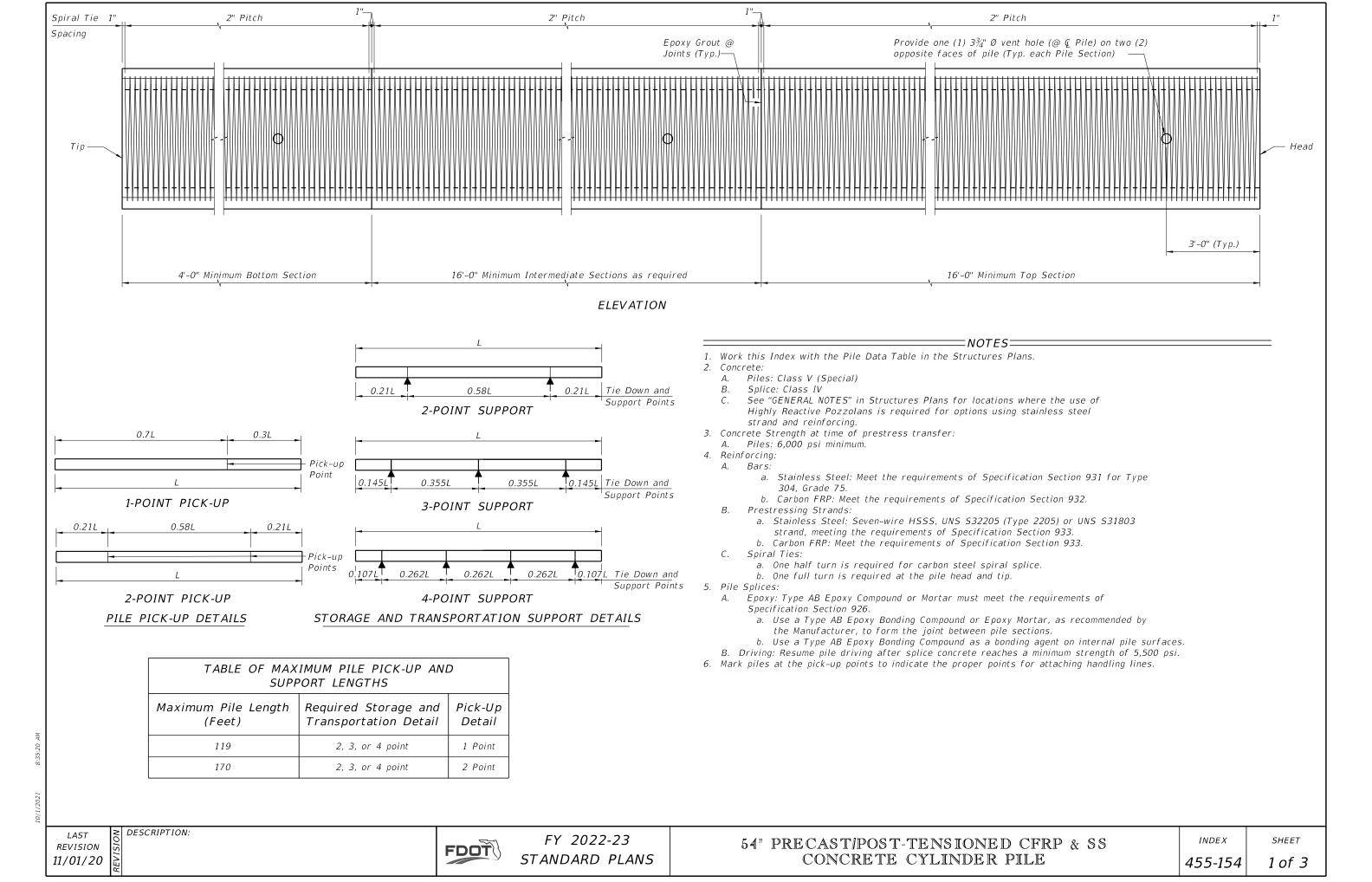
**REVISION** 11/01/20

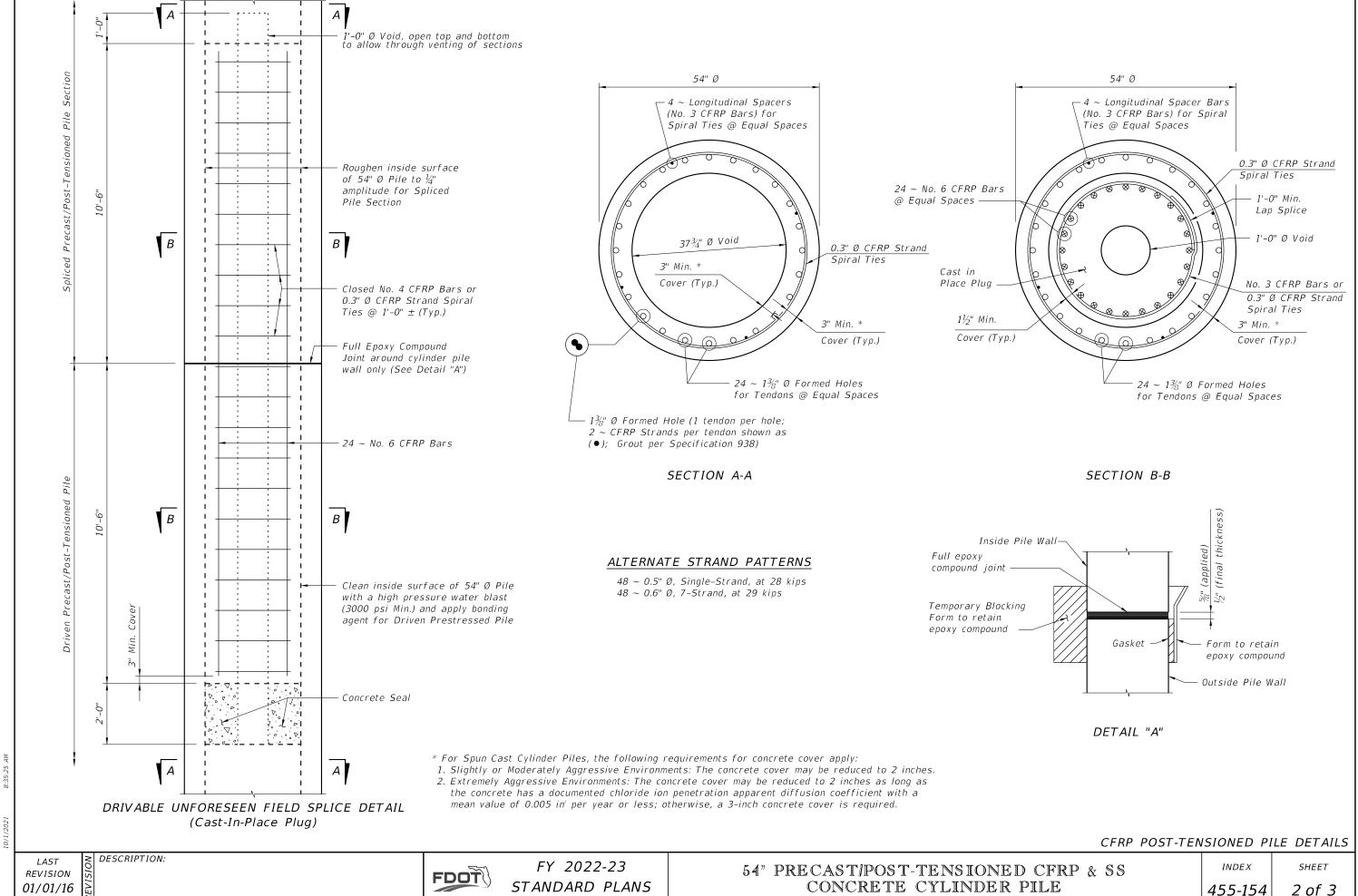
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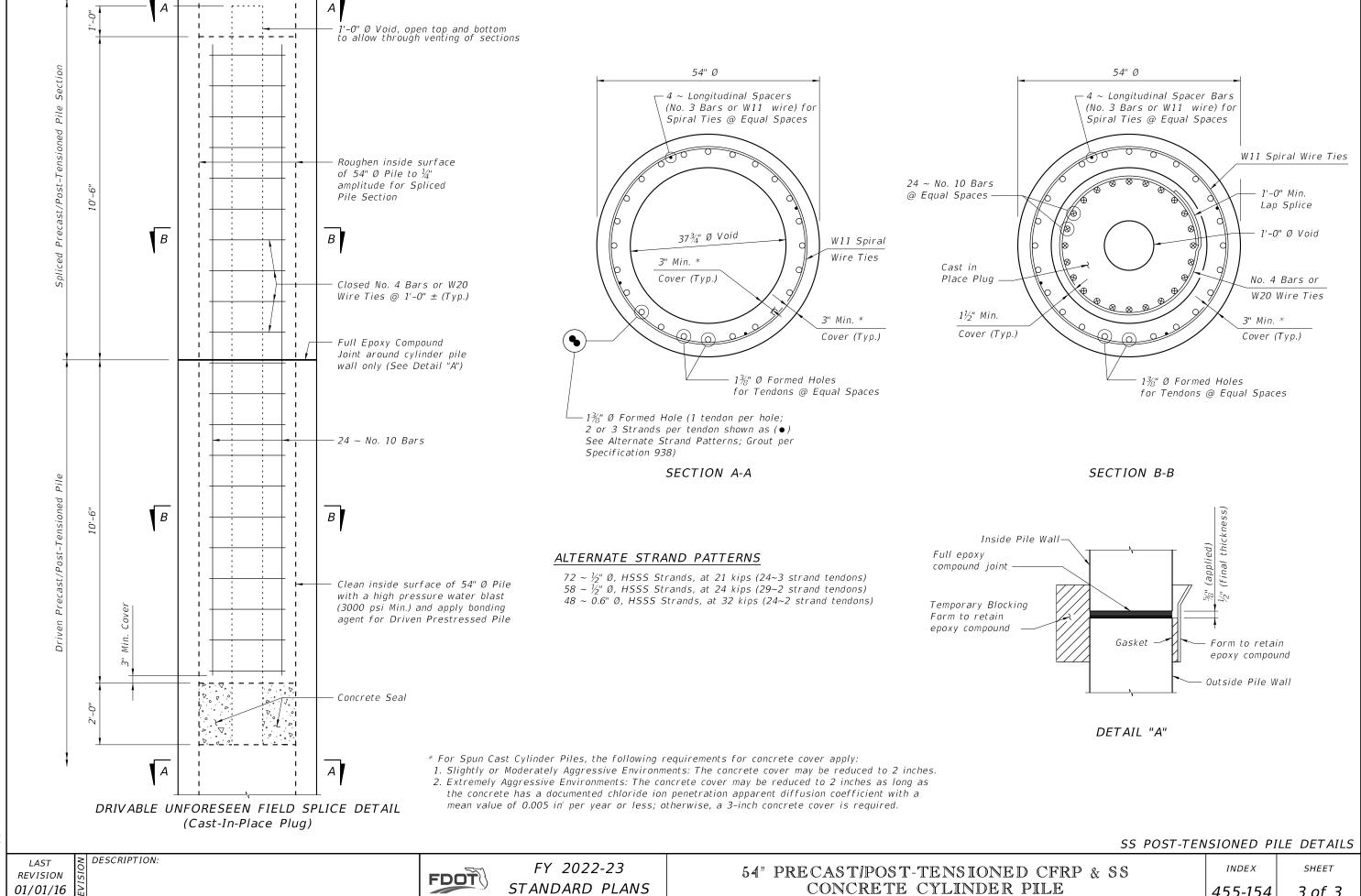
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FY 2022-23 STANDARD PLANS CONCRETE PILE

INDEX 455-130 SHEET





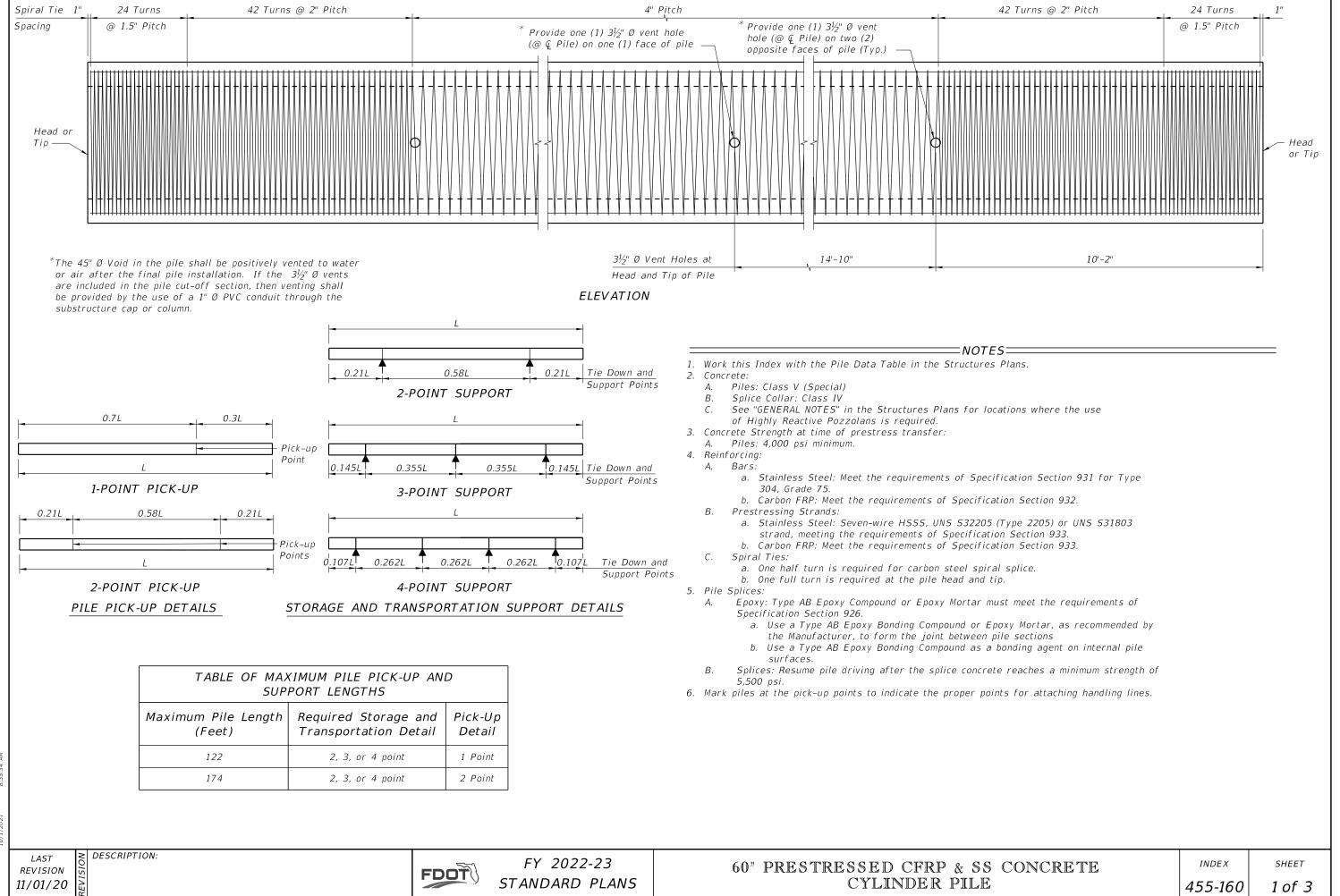


FDOT

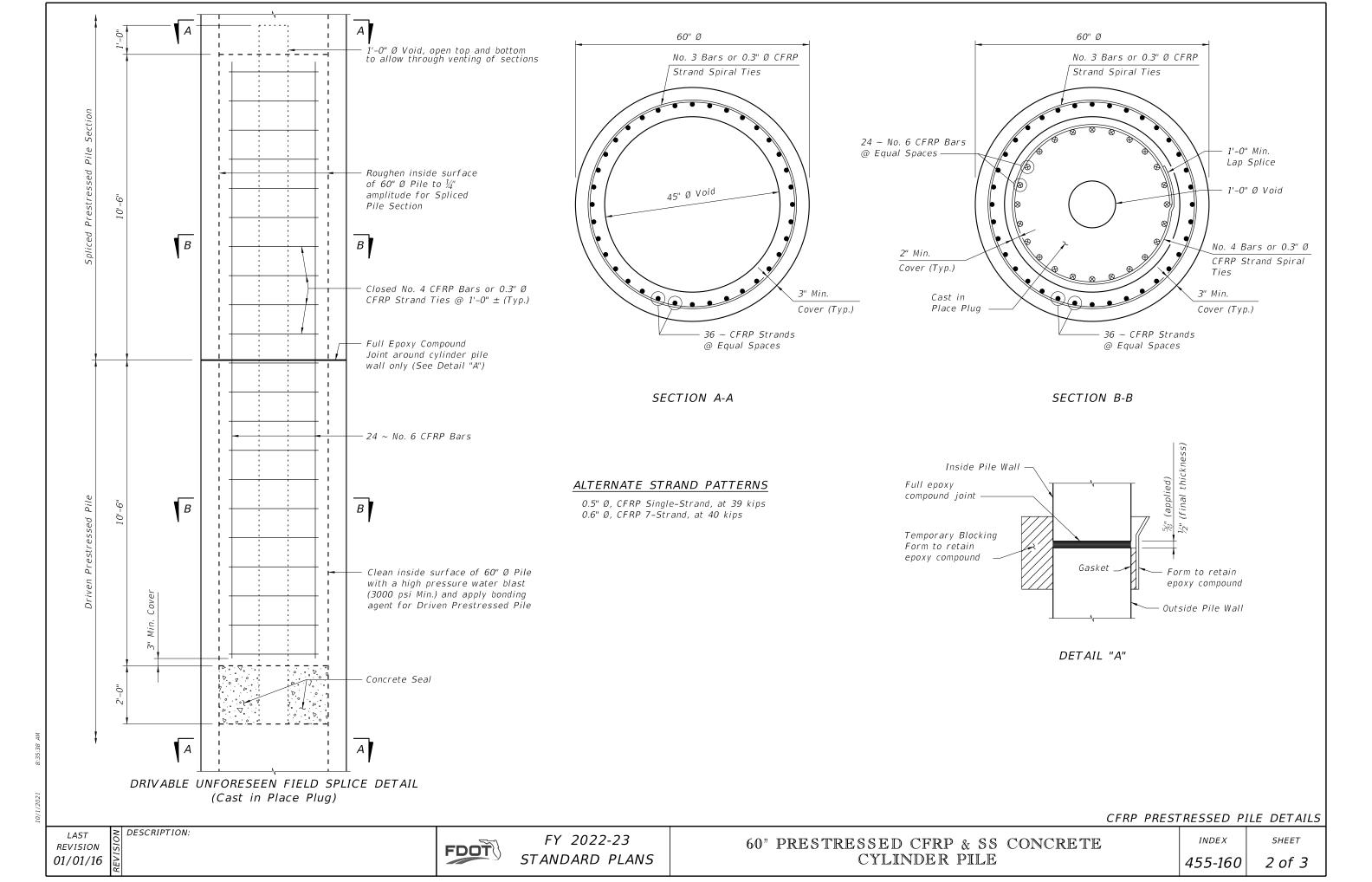
STANDARD PLANS

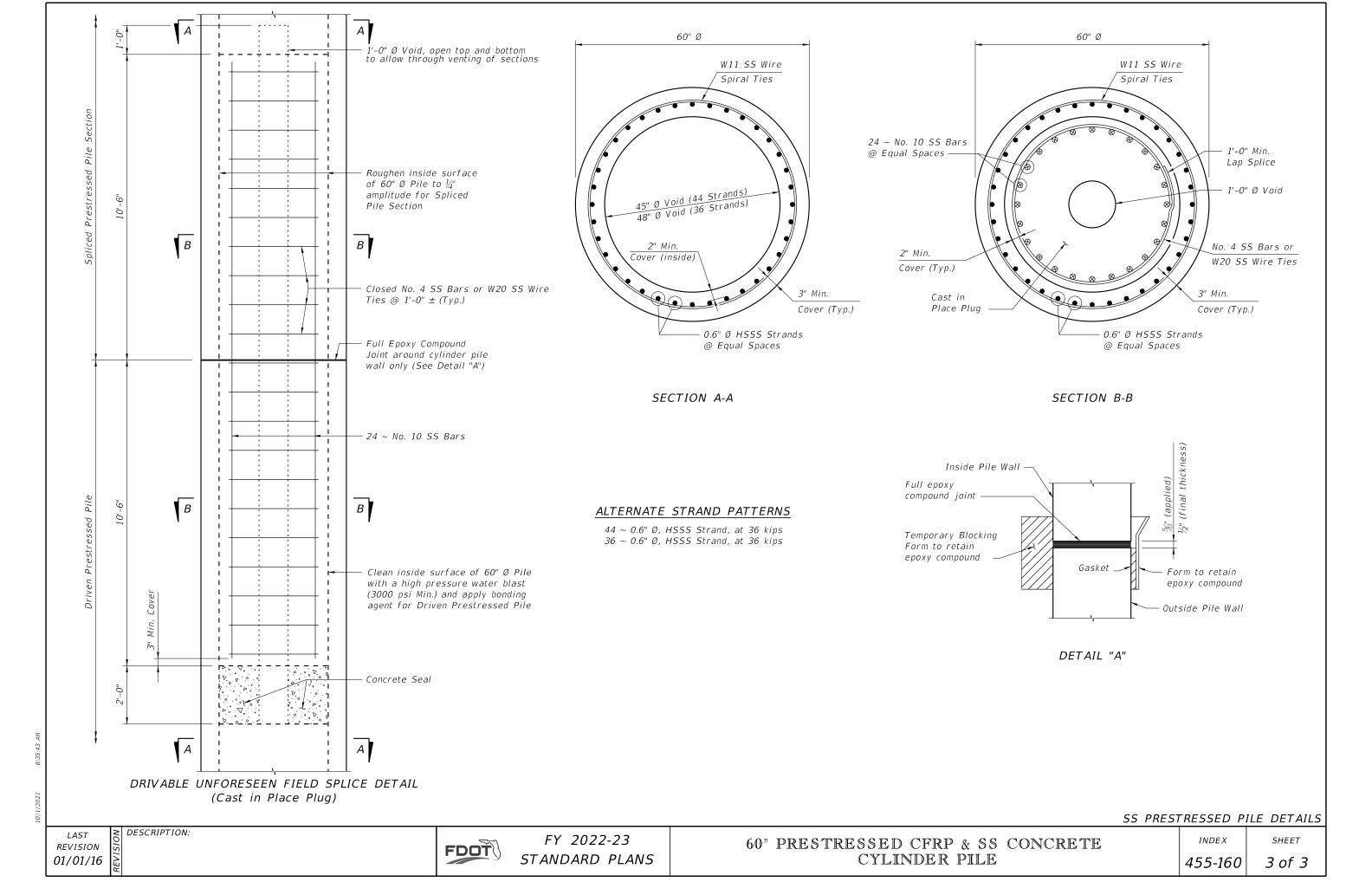
CONCRETE CYLINDER PILE

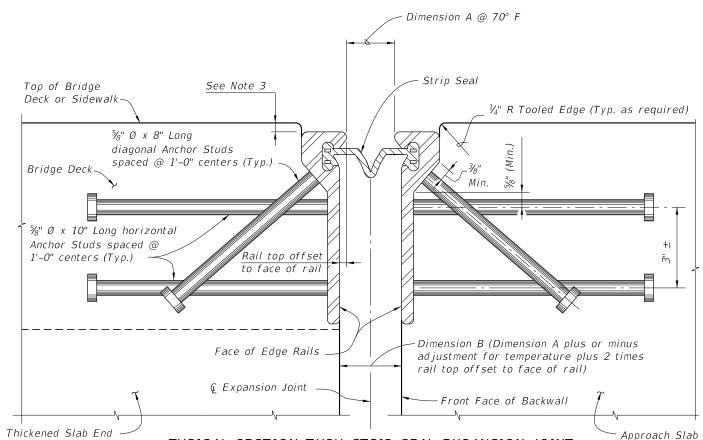
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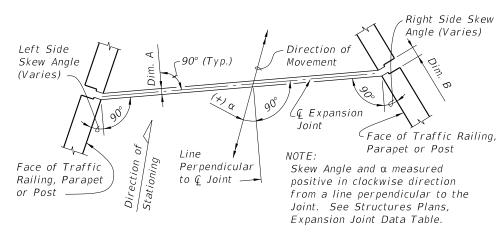
10/1/2021



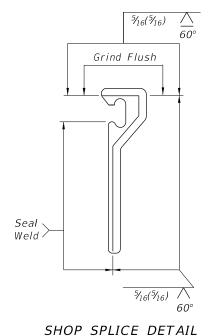




TYPICAL SECTION THRU STRIP SEAL EXPANSION JOINT (Begin or End Concrete Girder Bridge shown, Intermediate Supports and Steel Girder Bridge similar. Reinforcing Steel and Girder details not shown for clarity.)



MOVEMENT SCHEMATIC



# GENERAL NOTES:

- 1. Furnish Strip Seal Expansion Joint Systems in accordance with Specification Section 458.
- 2. Shape of Edge Rail shown is representative, minor variations depending on manufacturer
- 3. Recess the Edge Rail below the concrete surface in accordance with Specification Section 458.
- 4. Refer to the Expansion Joint Data Table in the Structures Plans for joint movement and Dimension A.
- 5. Refer to Specification Section 458 for installation and fabrication requirements.

DESCRIPTION:

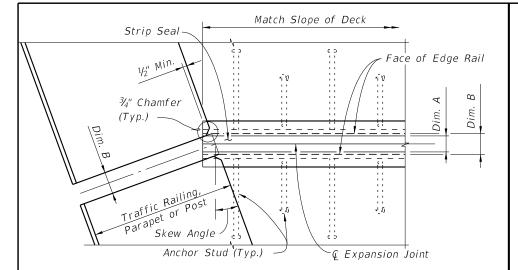
FDOT

FY 2022-23 STANDARD PLANS EXPANSION JOINT SYSTEM -

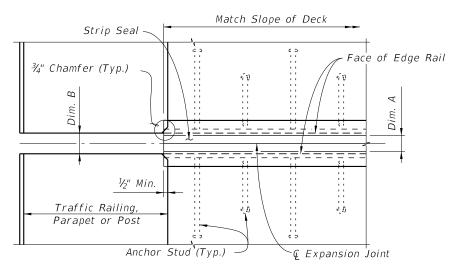
INDEX 458-100

SHEET 1 of 3

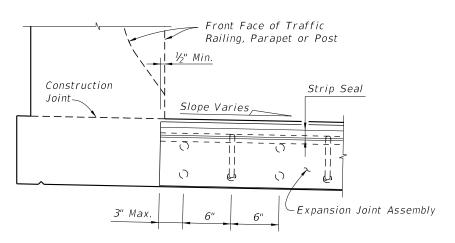
**REVISION** 11/01/19



# PARTIAL PLAN VIEW OF SKEWED JOINTS



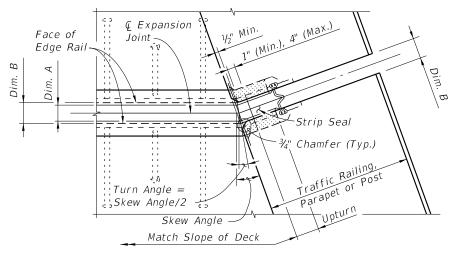
# PARTIAL PLAN VIEW OF NONSKEWED JOINTS



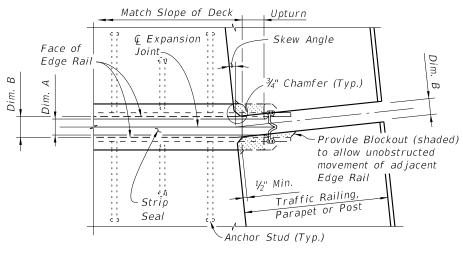
PARTIAL SECTION ALONG G JOINT

\_\_\_\_ JOINT TREATMENT AT HIGH SIDE OF DECK \_\_\_\_\_ WITH SLOPE ≥ 1%

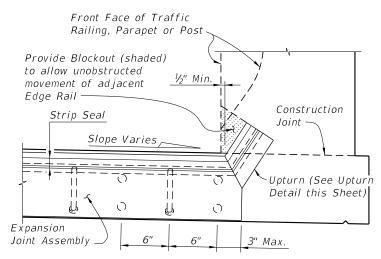
(Sidewalk Cover Plate where applicable not shown for clarity)



# PARTIAL PLAN VIEW OF JOINTS SKEWED GREATER THAN 6°



# PARTIAL PLAN VIEW OF NONSKEWED JOINTS & JOINTS SKEWED 6° OR LESS

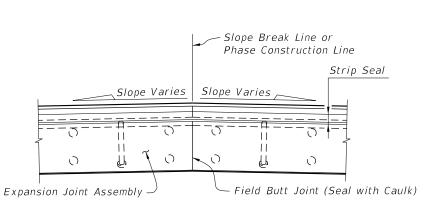


PARTIAL SECTION ALONG Q JOINT

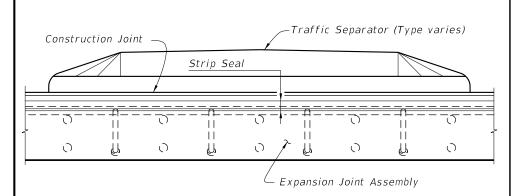
JOINT TREATMENT AT LOW SIDE OF DECK &

HIGH SIDE OF DECK WITH SLOPE < 1%

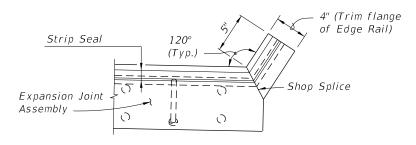
(Sidewalk Cover Plate where applicable not shown for clarity)



PARTIAL SECTION ALONG Q JOINT AT FIELD BUTT JOINT LOCATION (CROWNED DECK OR SLAB SHOWN)



PARTIAL SECTION ALONG Q JOINT THRU TRAFFIC SEPARATOR



UPTURN DETAIL
(TYPICAL AT TRAFFIC BARRIERS AND PARAPETS)

LAST REVISION 11/01/19

DESCRIPTION:

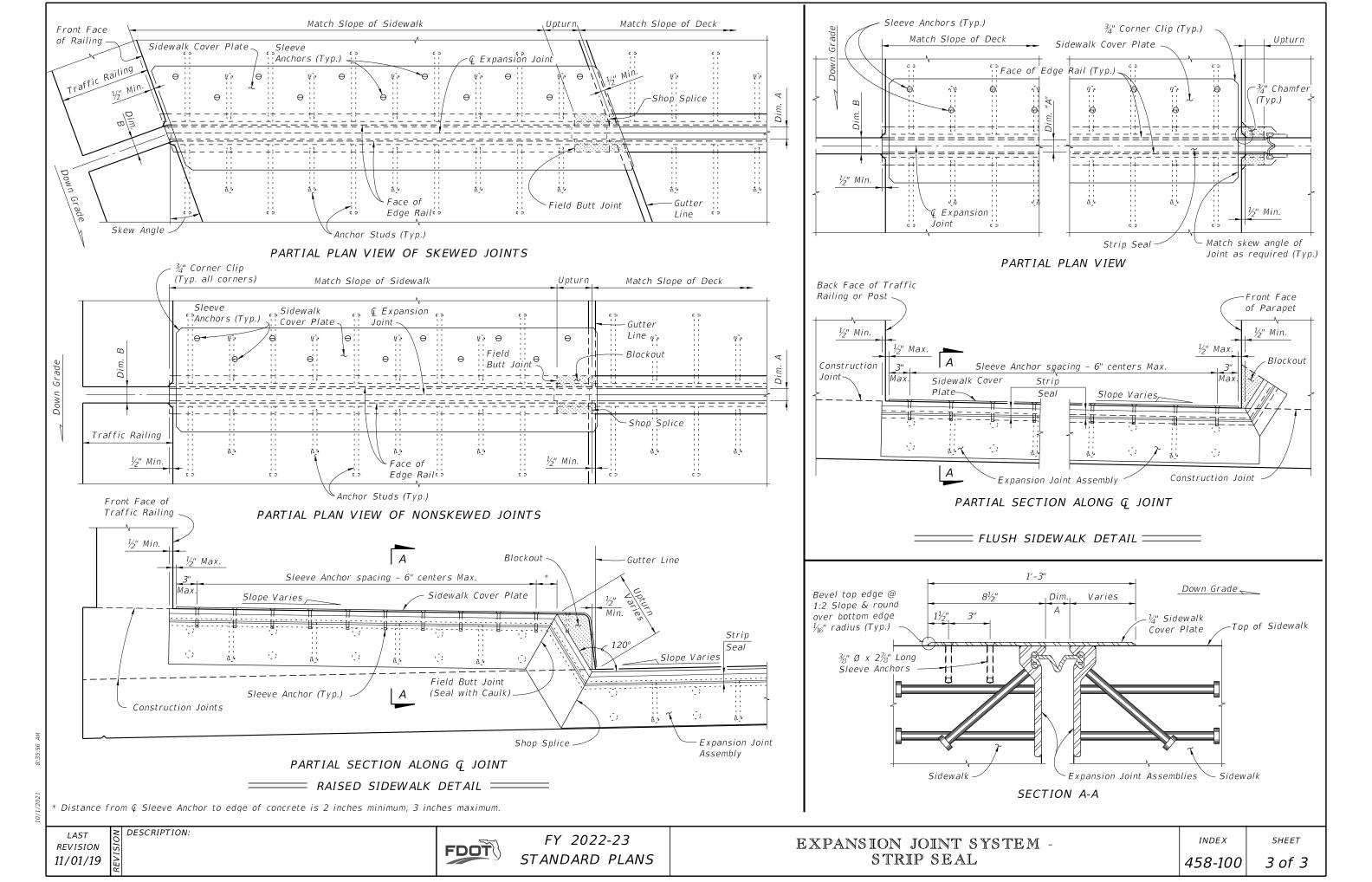
FDOT

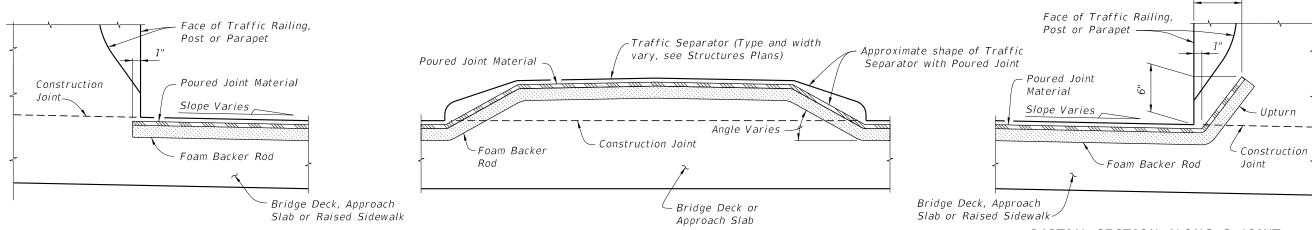
FY 2022-23 STANDARD PLANS EXPANSION JOINT SYSTEM - STRIP SEAL

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SHEET

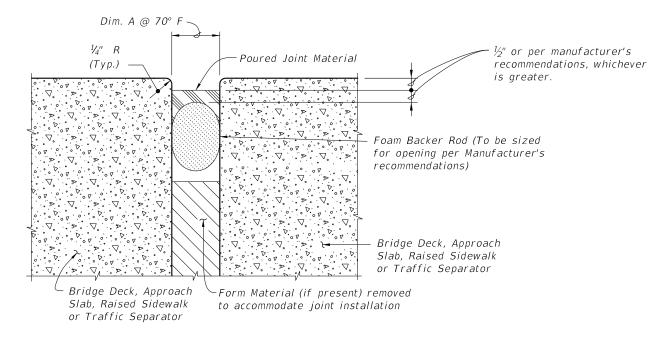
458-100 2 of 3





PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPES 1% OR GREATER

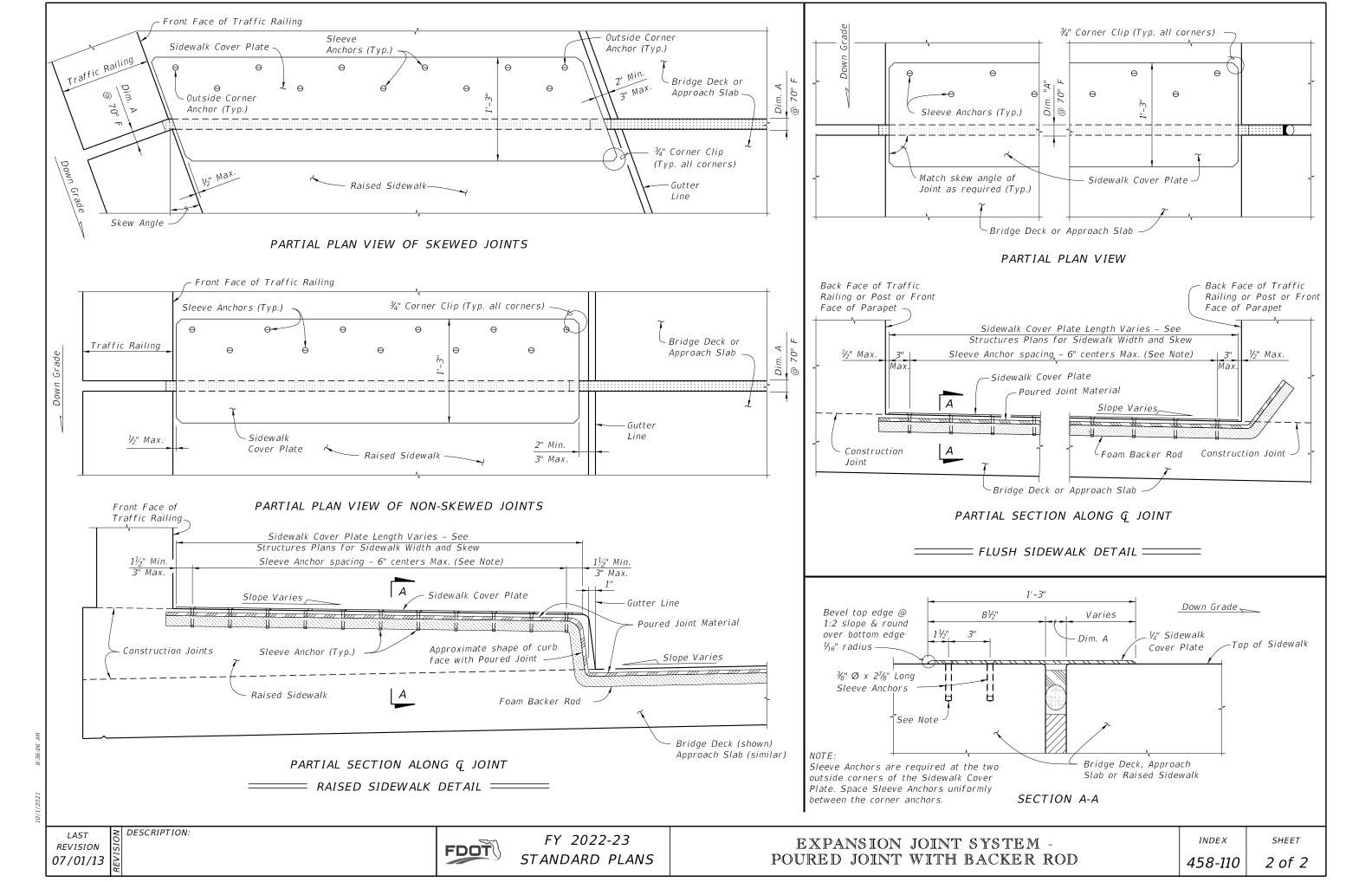
PARTIAL SECTION ALONG Q JOINT, JOINT TREATMENT AT TRAFFIC SEPARATOR PARTIAL SECTION ALONG Q JOINT
JOINT TREATMENT AT LOW SIDE OF DECK OR
HIGH SIDE OF DECK WITH SLOPES < 1%

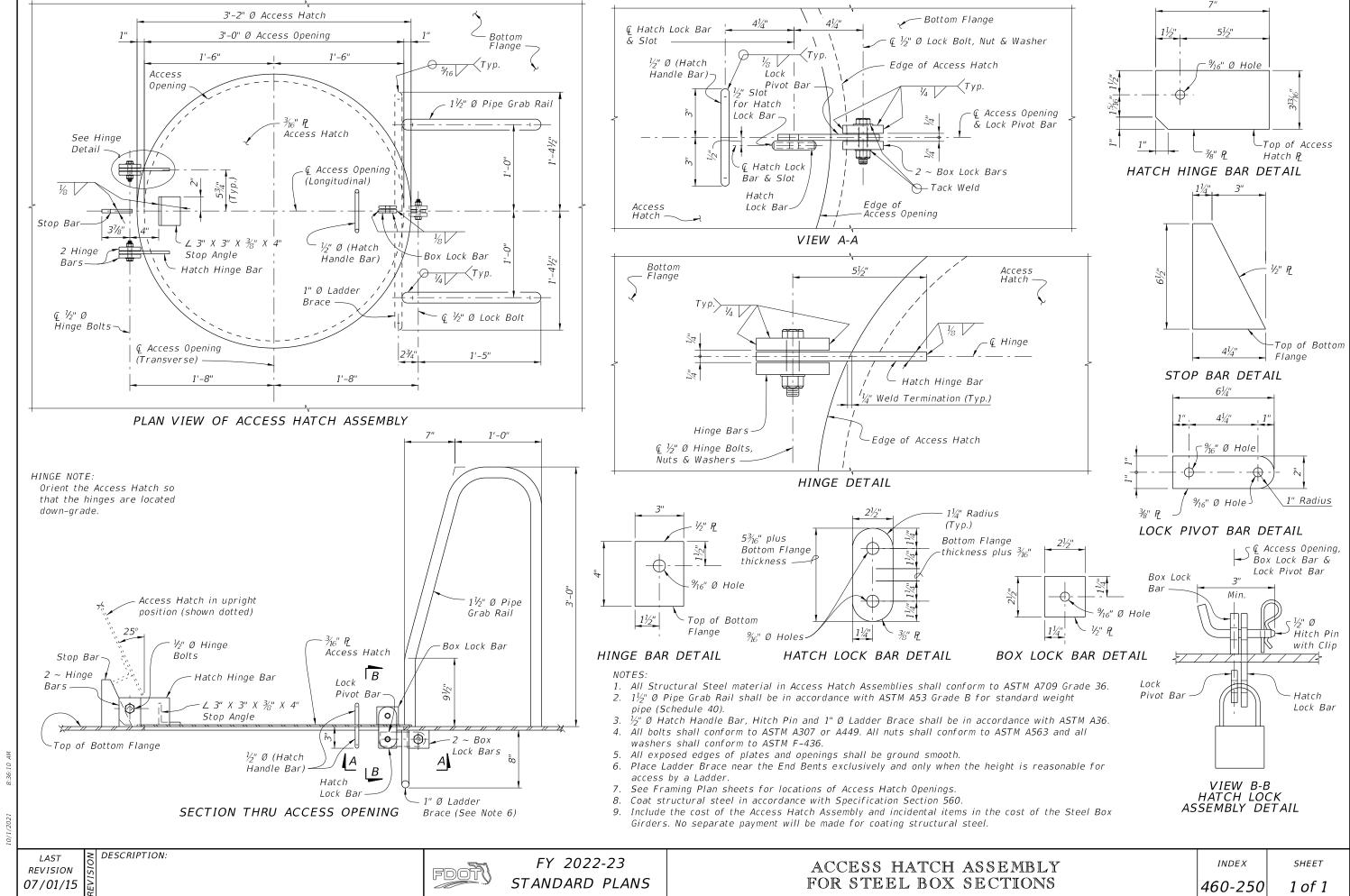


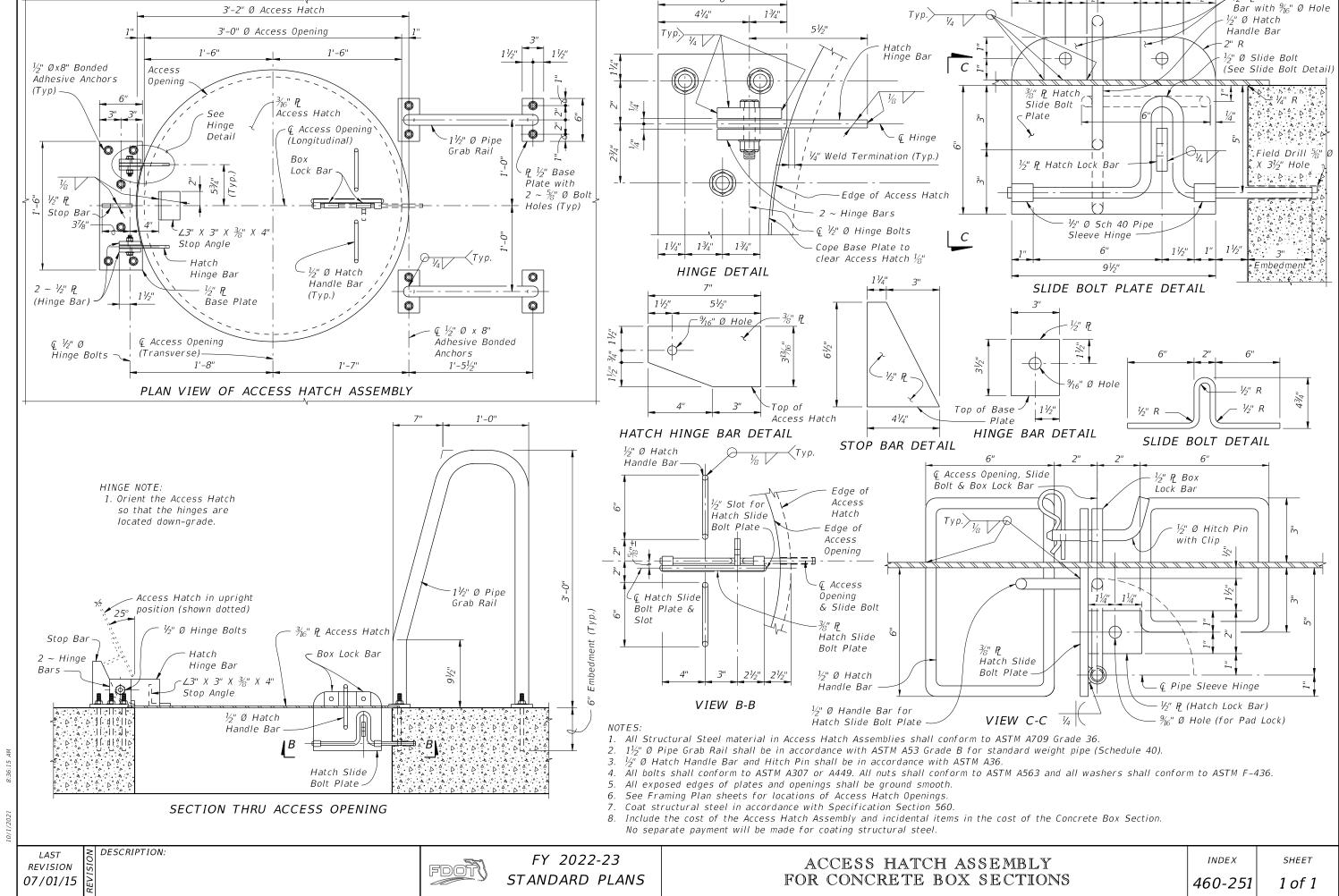
# TYPICAL SECTION THRU JOINT

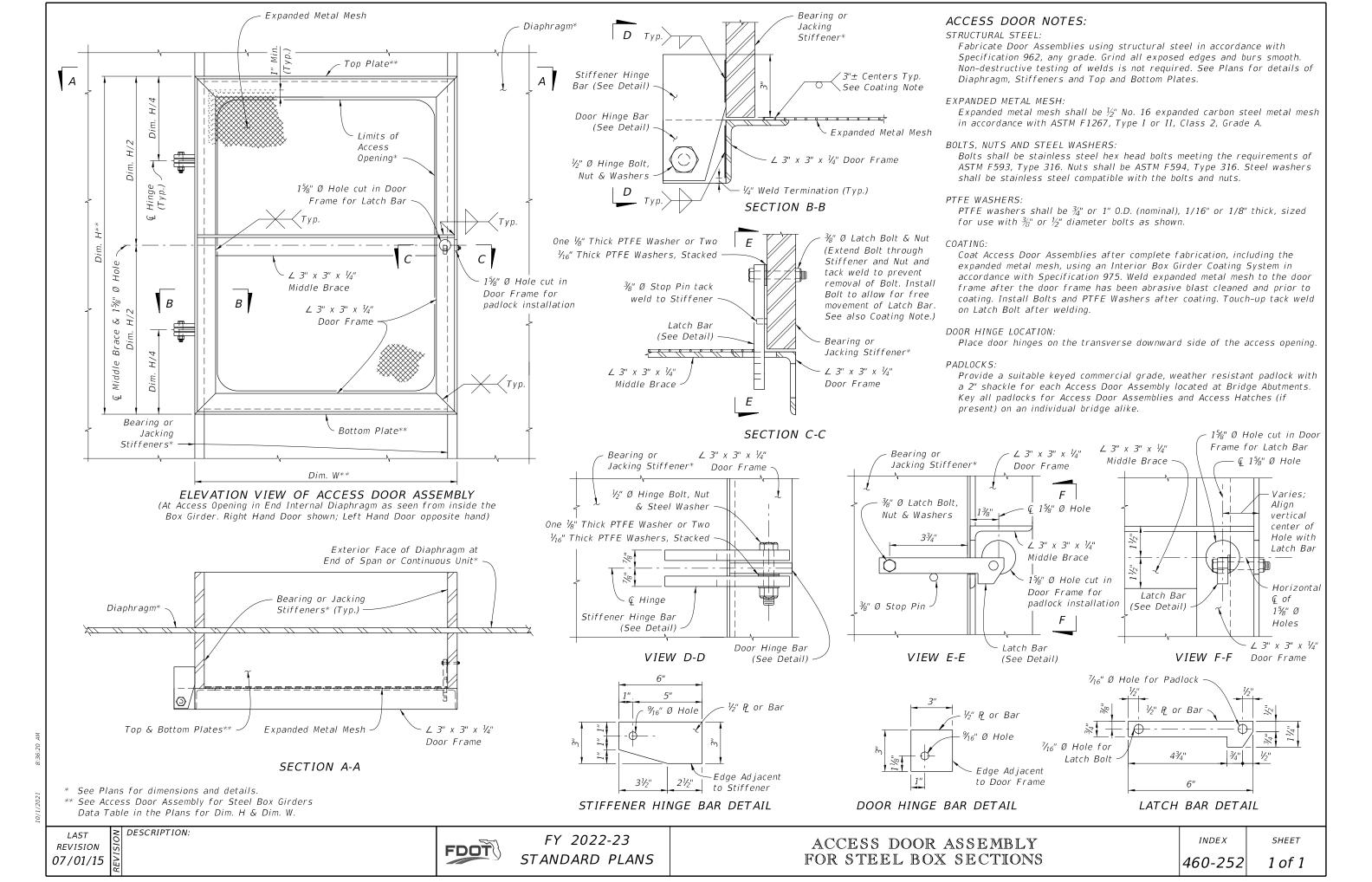
## GENERAL NOTES:

- 1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
- 2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.









This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be 3'" by 21'/" slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM A449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for  $\frac{1}{4}$ " Ø anchor bolts; 55,000 lbs. for the  $\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the  $\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

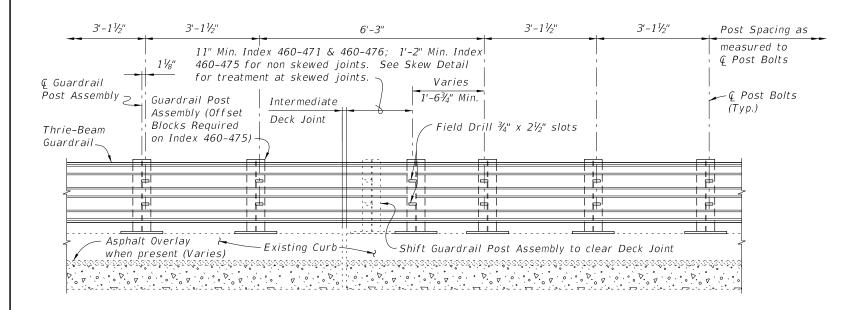
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1½" spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

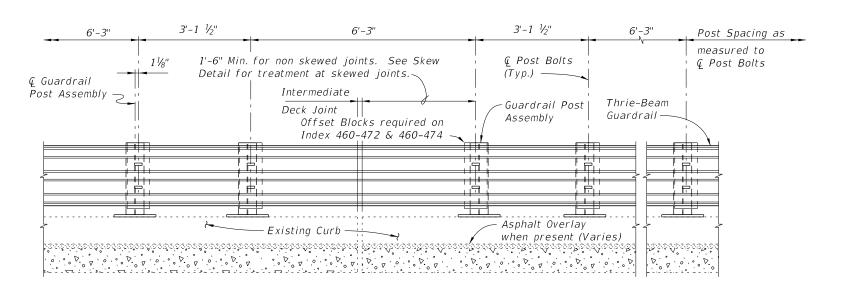
- THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in  $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.
- BEARING PADS: Provide plain Neoprene pads with a durometer hardness of 60 or 70 and meeting the requirements of Specification Section 932, for ancillary structures.
- ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.
- BARRIER DELINEATORS: Install Barrier Delineators at the top of the guardrail offset blocks in accordance with Specification Section 705. Match the Barrier Delineators color (white or yellow) to the near edgeline.
- PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index 536-001 for details.
- BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

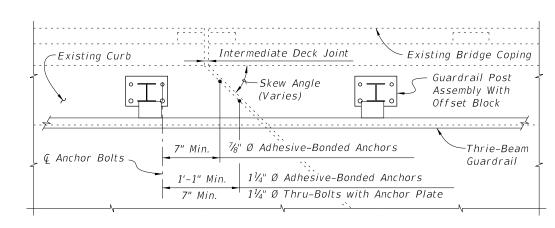
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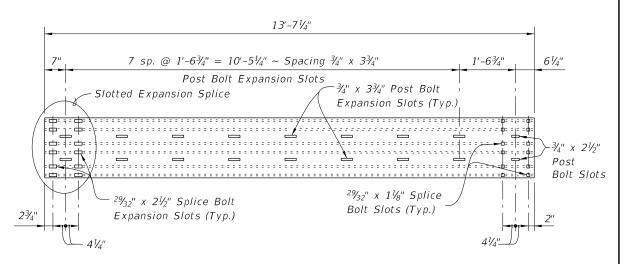
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL
FOR INDEX 460-471, 460-475 & 460-476



PARTIAL ELEVATION OF INSIDE FACE OF RAILING MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX 460-472, 460-473 & 460-474

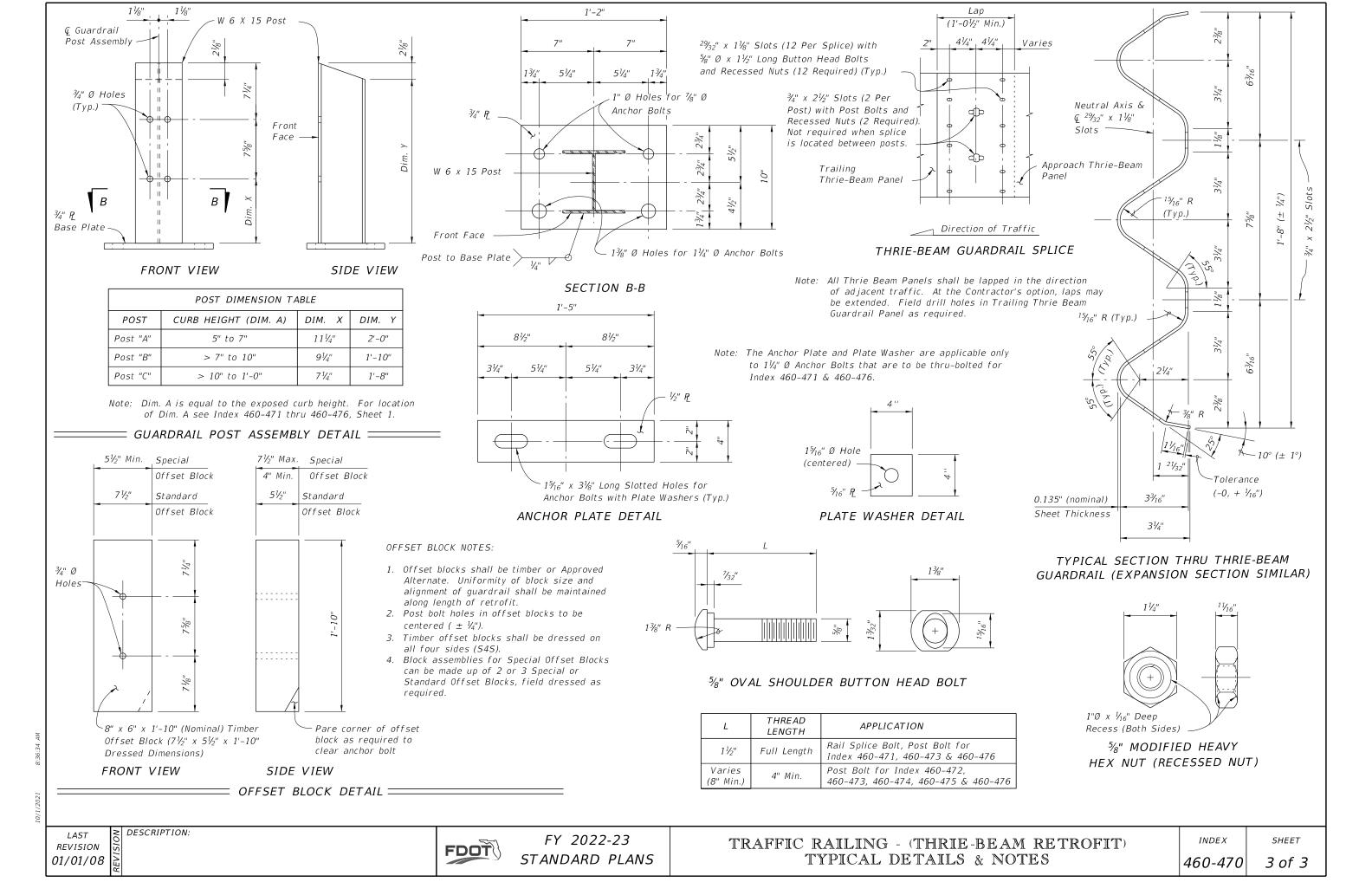


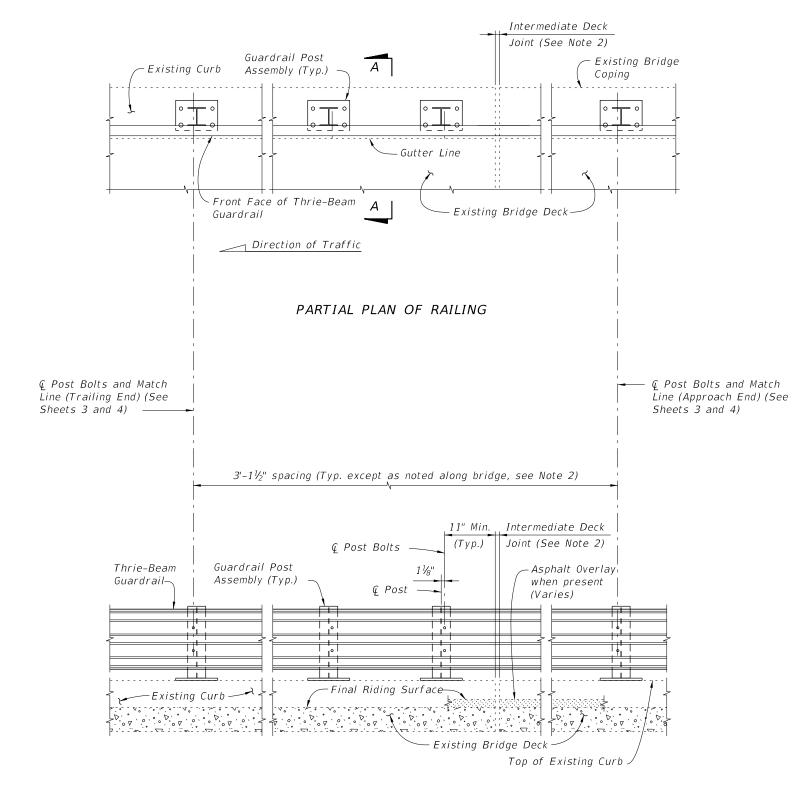
PARTIAL PLAN
INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION

DESCRIPTION:





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

#### NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

LAST REVISION 01/01/08

DESCRIPTION:

FDOT

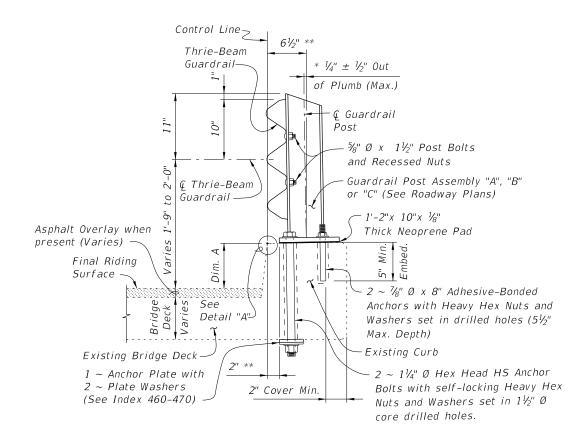
FY 2022-23
STANDARD PLANS

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)
NARROW CURB

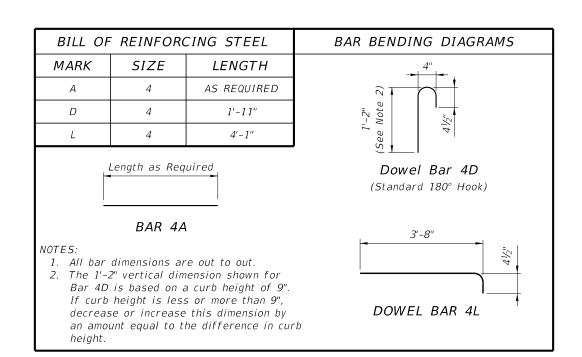
INDEX

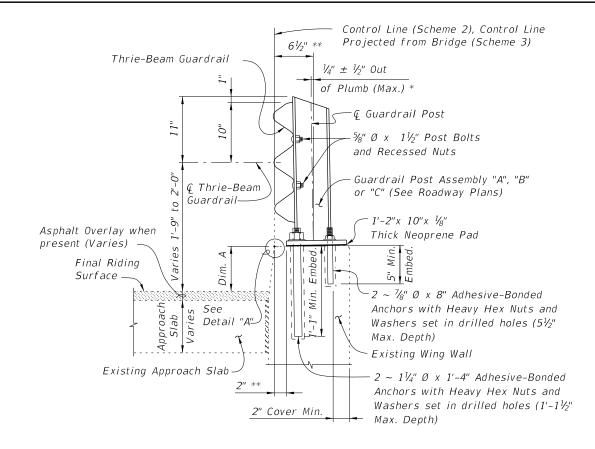
SHEET

460-471 1 of 4



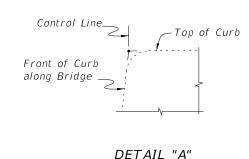
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

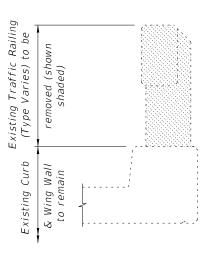




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

#### CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

REVISION 01/01/08

**FDOT** 

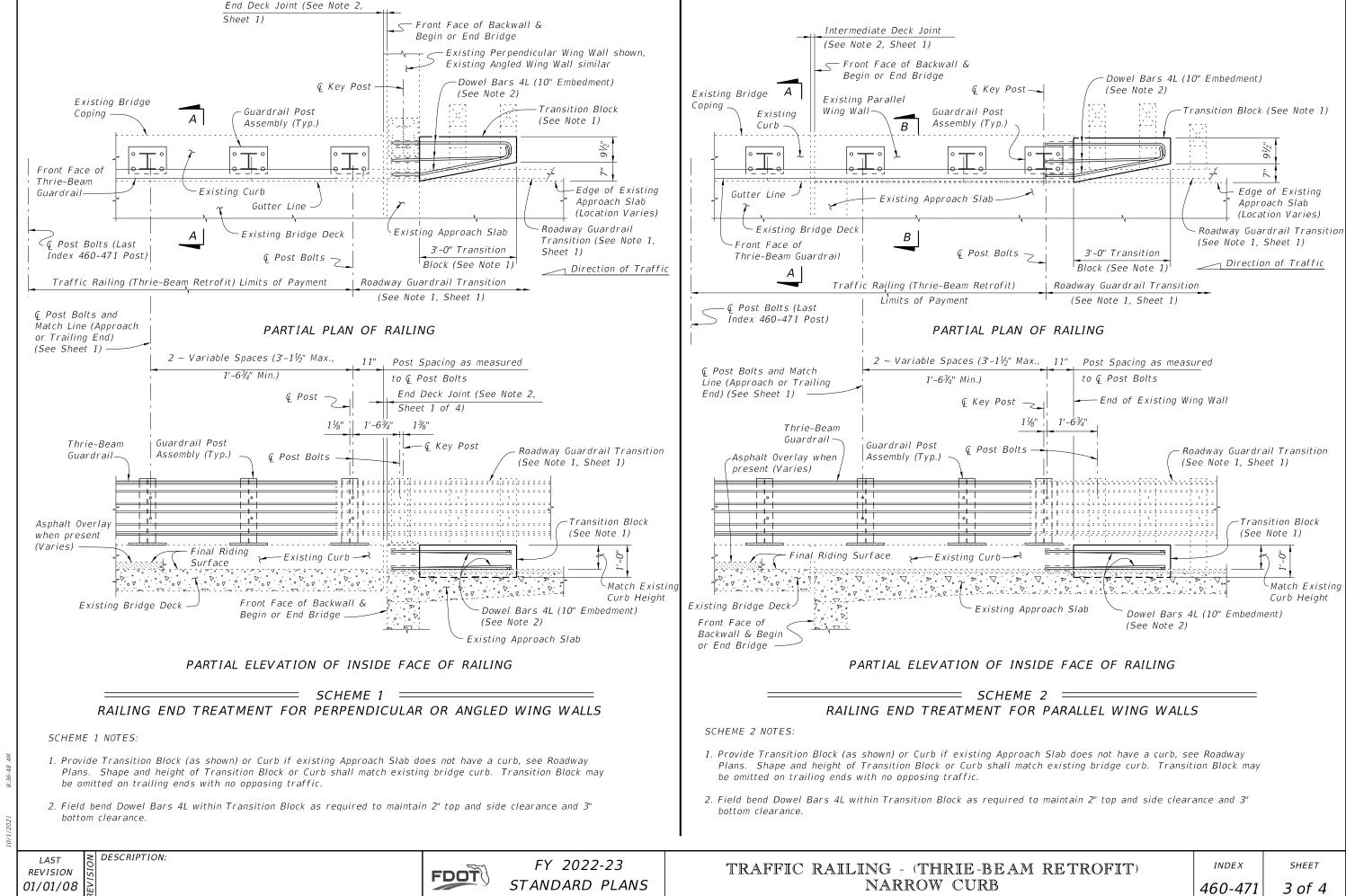
FY 2022-23 STANDARD PLANS

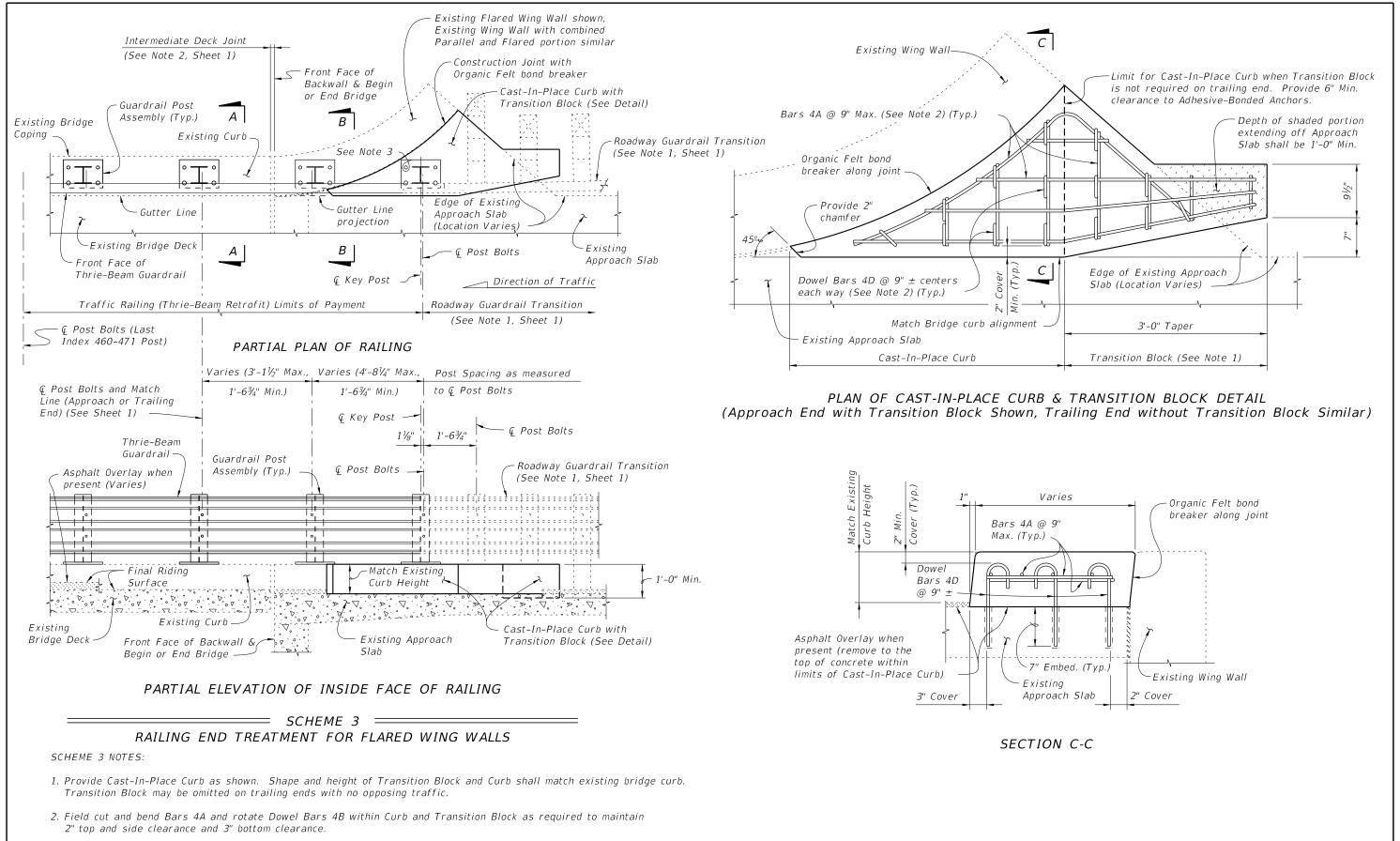
TRAFFIC RAILING - (THRIE-BEAM RETROFIT)

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SHEET

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REVISION 11/01/16

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

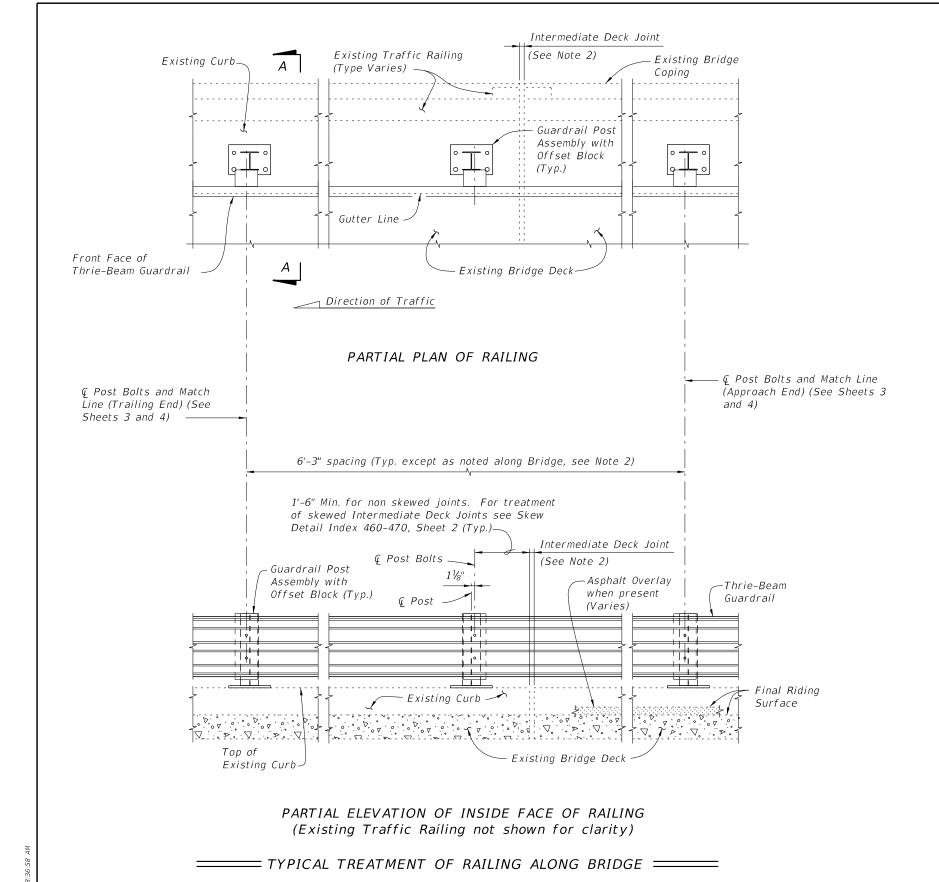
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

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460-471

3. A single  $\frac{7}{8}$  Ø x 8" Adhesive-Bonded Anchor may be omitted as shown when 2" clear cover cannot be provided.



## NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2.

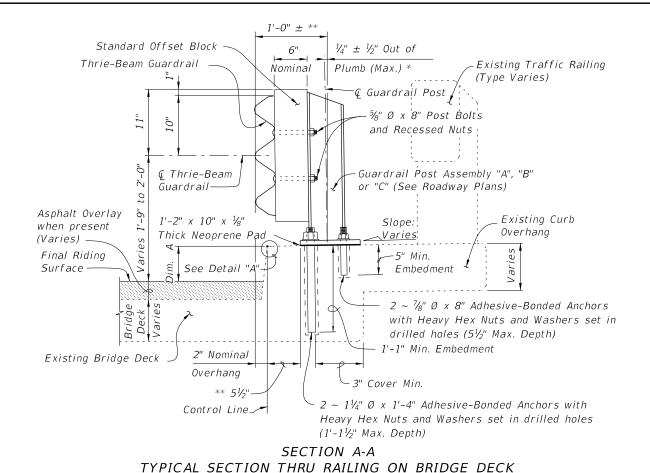
For Traffic Railing Notes and Details see Index 460-470.

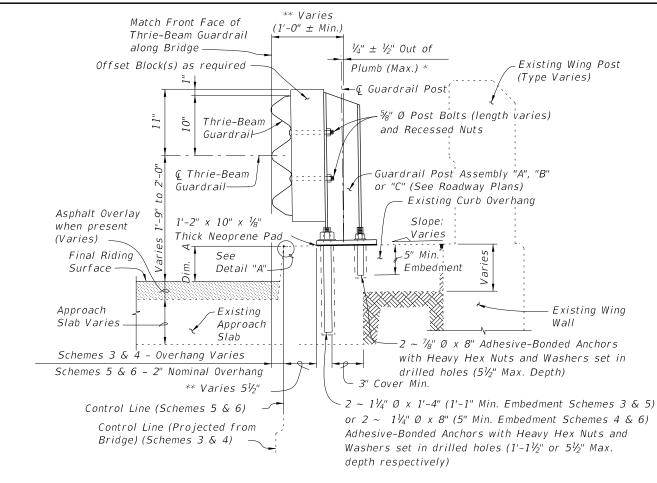
**REVISION** 01/01/08

DESCRIPTION:

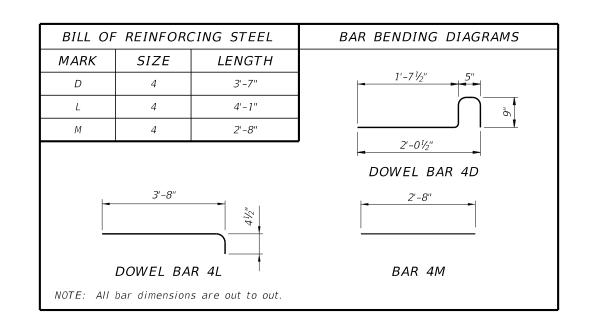
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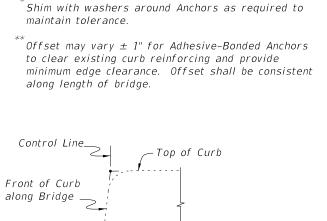
FY 2022-23 STANDARD PLANS



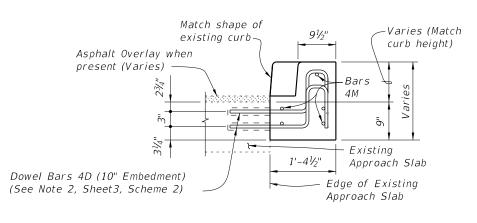


SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)





DETAIL "A"



VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

**REVISION** 07/01/08

**FDOT** 

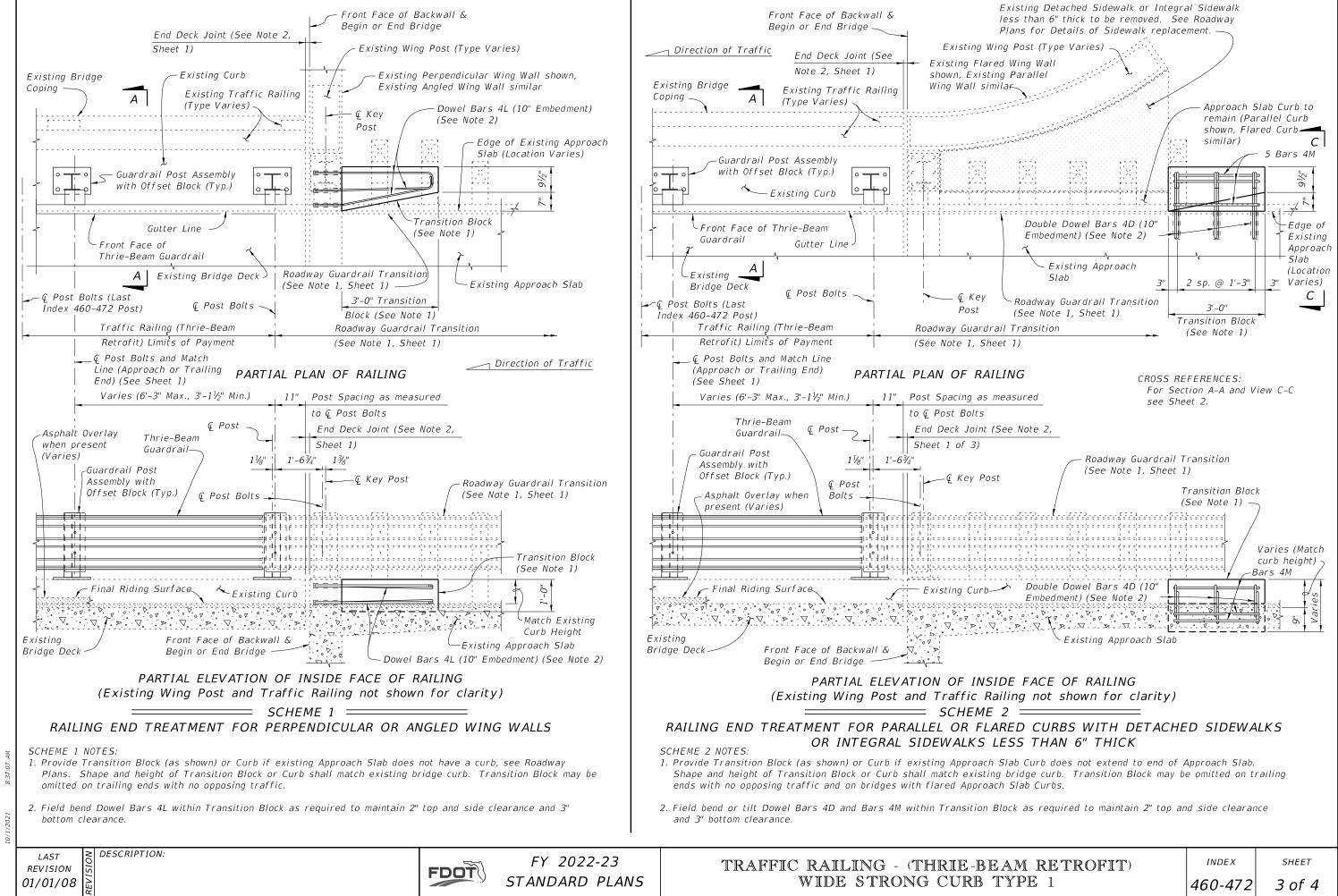
FY 2022-23 STANDARD PLANS

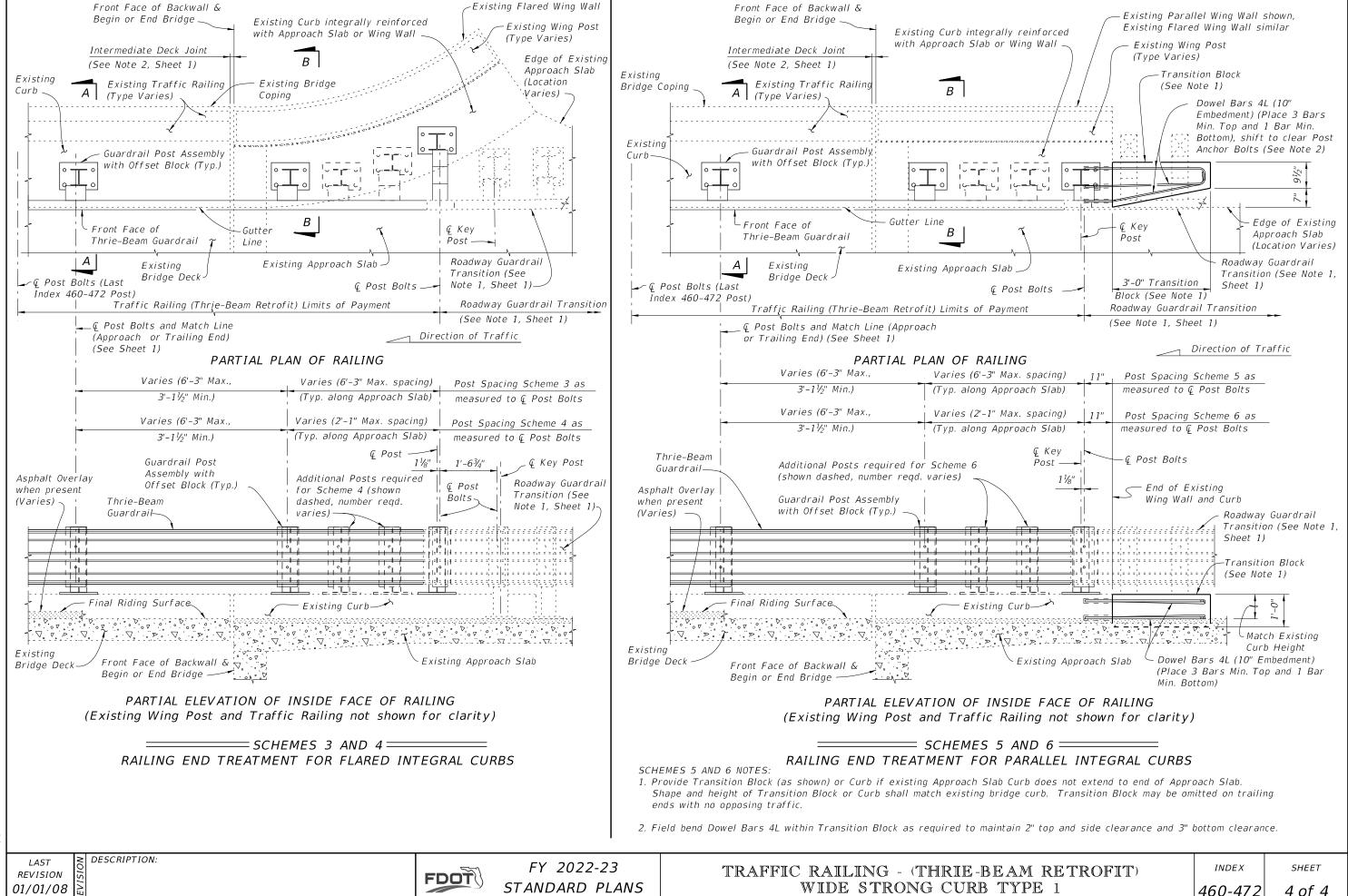
WIDE STRONG CURB TYPE 1

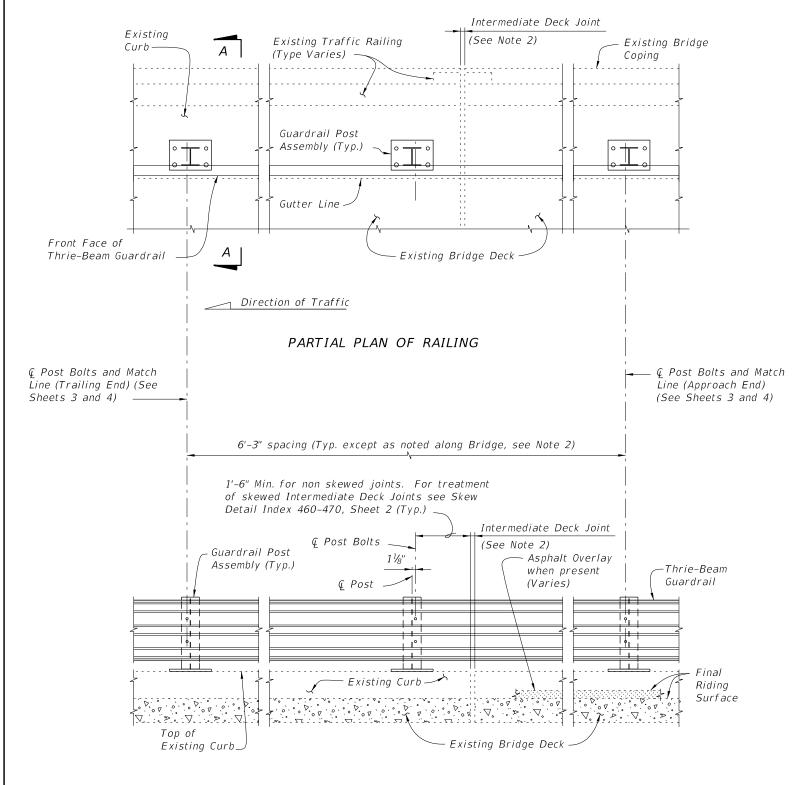
INDEX 460-472

SHEET

DESCRIPTION:







PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

#### NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

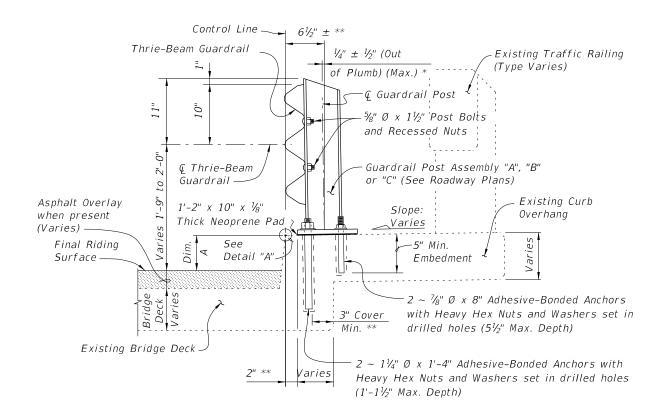
CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

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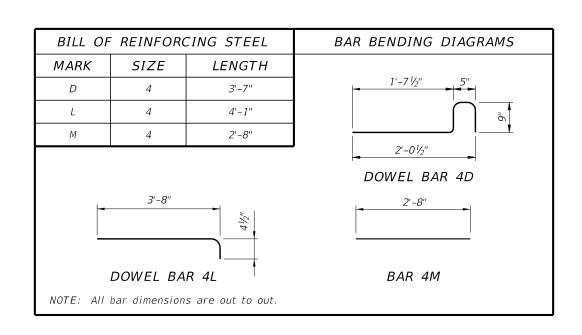
LAST REVISION 01/01/08

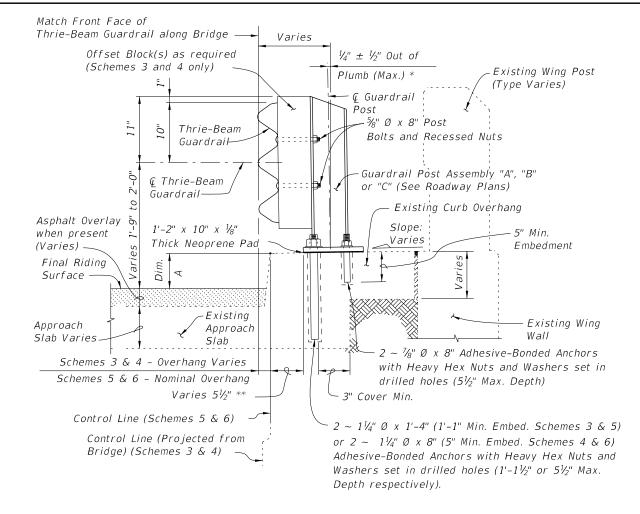
DESCRIPTION:





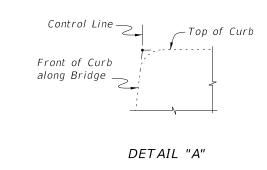
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

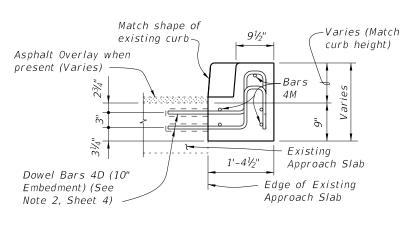




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- \* Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- \*\* Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





## VIEW C-C

# CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index 460-470.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

DESCRIPTION: REVISION 07/01/08

**FDOT** 

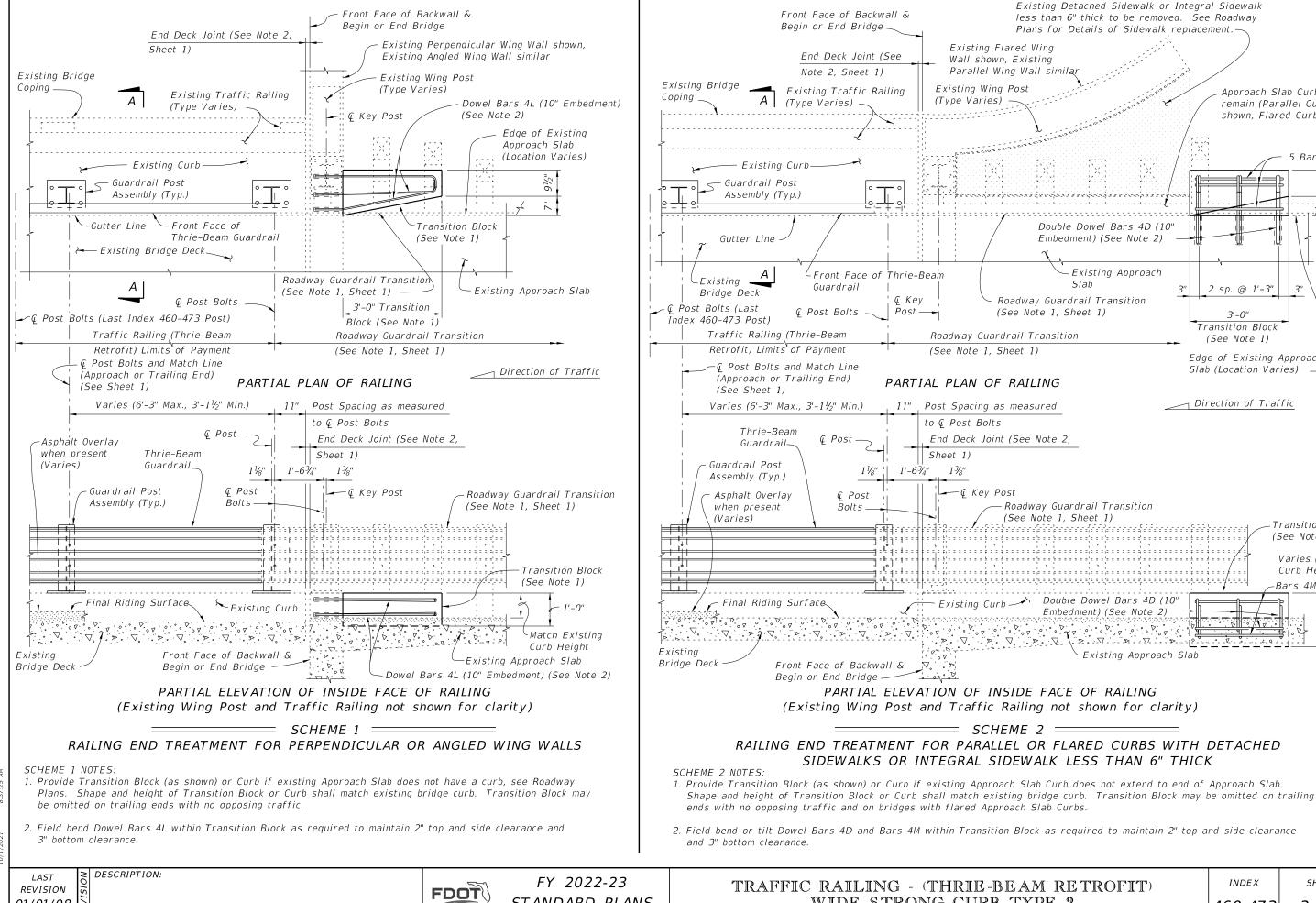
FY 2022-23 STANDARD PLANS

WIDE STRONG CURB TYPE 2

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STANDARD PLANS

WIDE STRONG CURB TYPE 2

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Approach Slab Curb to

shown, Flared Curb similar,

С

5 Bars 4M

Transition Block

Varies (Match Curb Height) -

(See Note 1)

-Bars 4M

remain (Parallel Curb

2 sp. @ 1'-3"

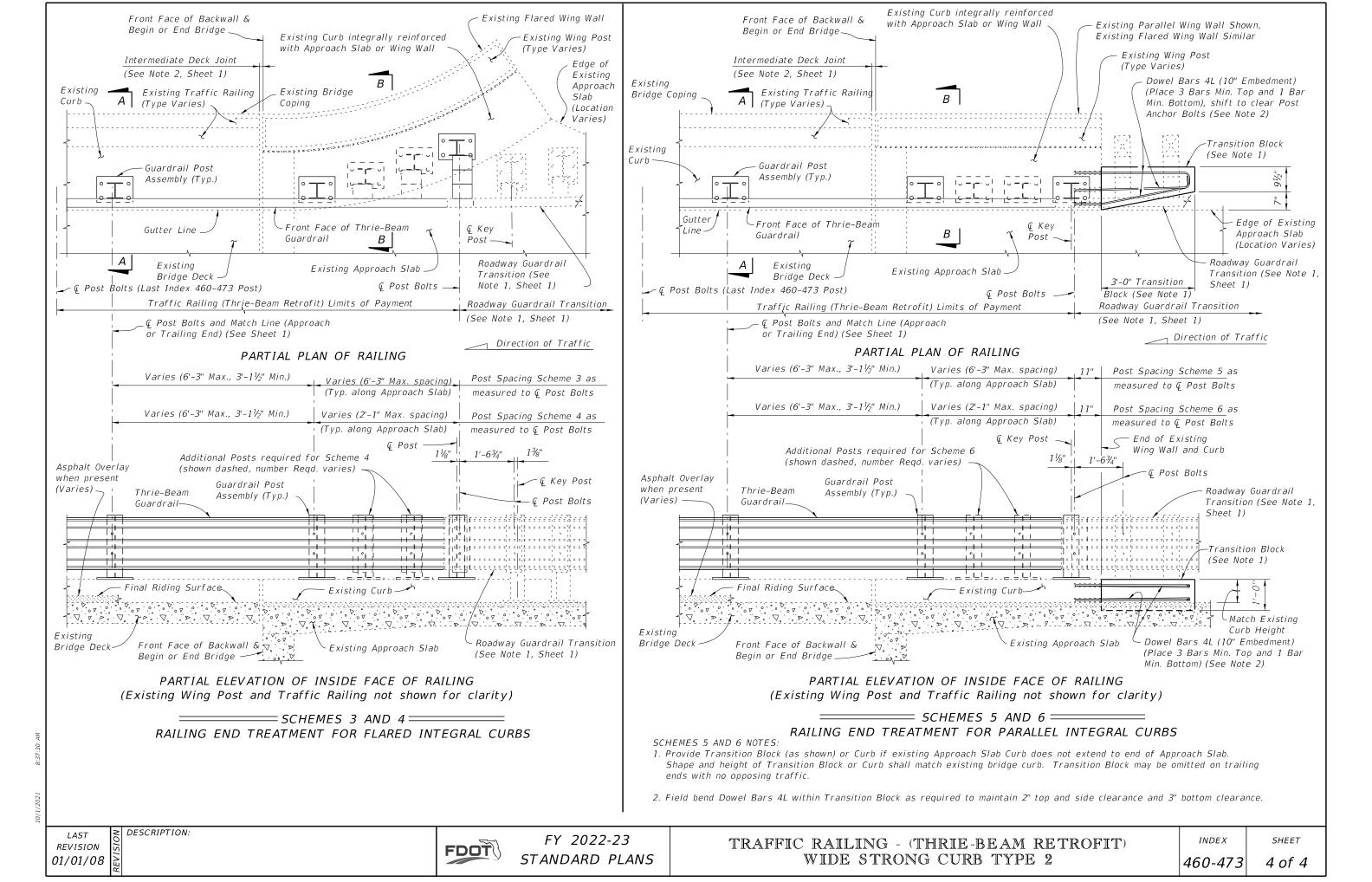
3'-0"

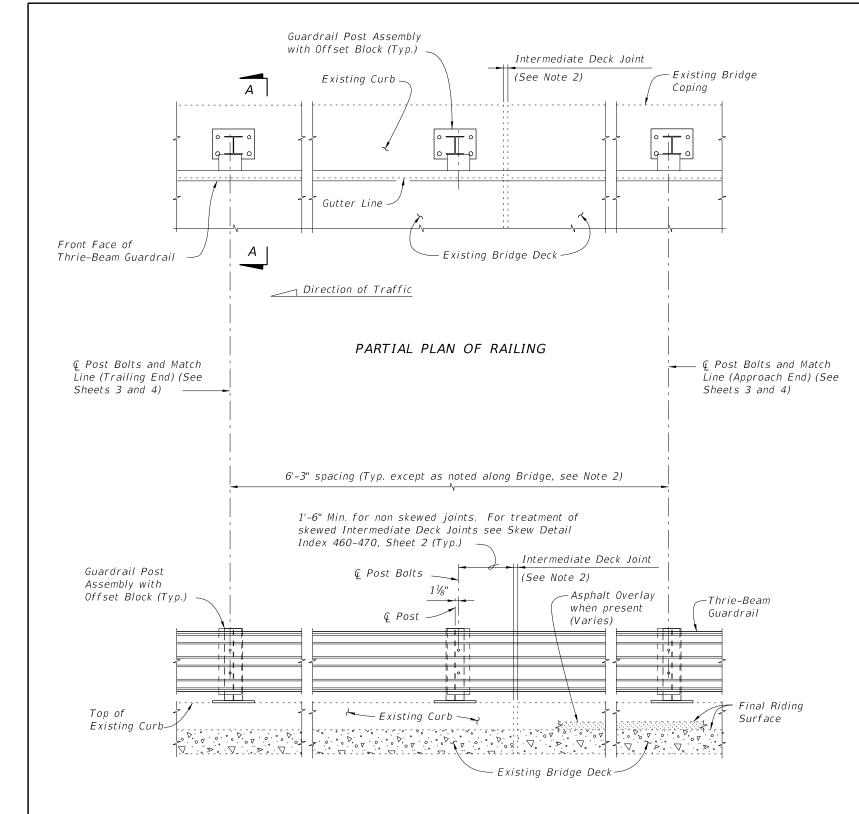
Transition Block

(See Note 1)

Edge of Existing Approach

Slab (Location Varies)





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

===== TYPICAL TREATMENT OF RAILING ALONG BRIDGE =======

#### NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2.

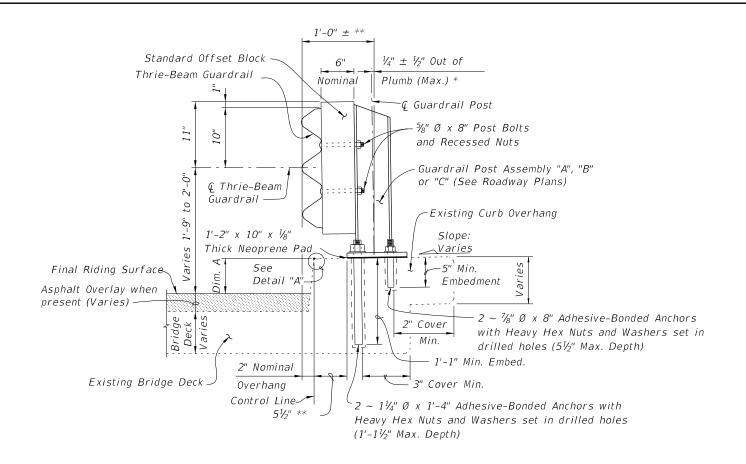
For Traffic Railing Notes and Details see Index 460-470.

**REVISION** 01/01/08

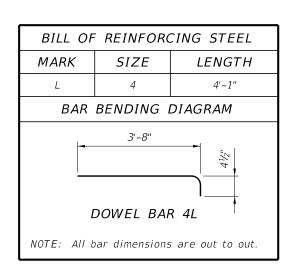
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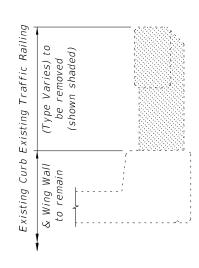
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FY 2022-23 STANDARD PLANS

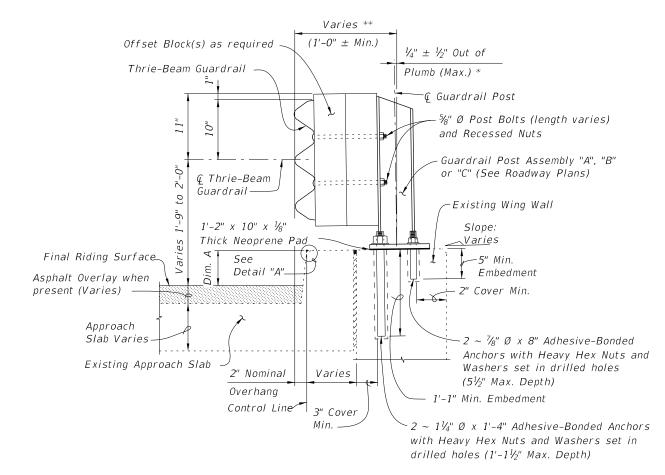


SECTION A-A
TYPICAL SECTION THRU RAILING ON BRIDGE DECK





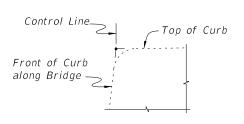
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



# SECTION B-B (SCHEME 2) TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

\*Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

\*\*Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



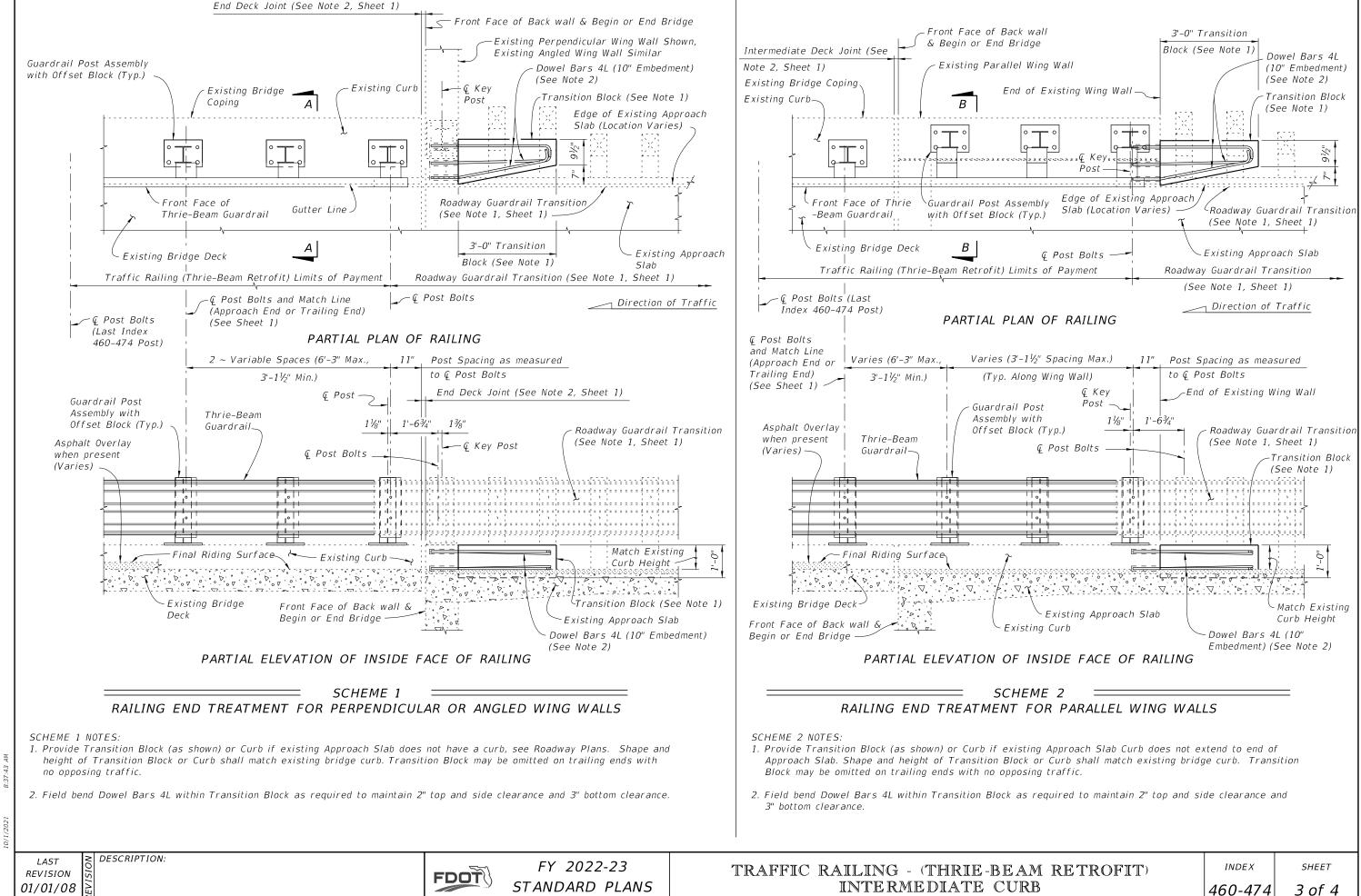
DETAIL "A"

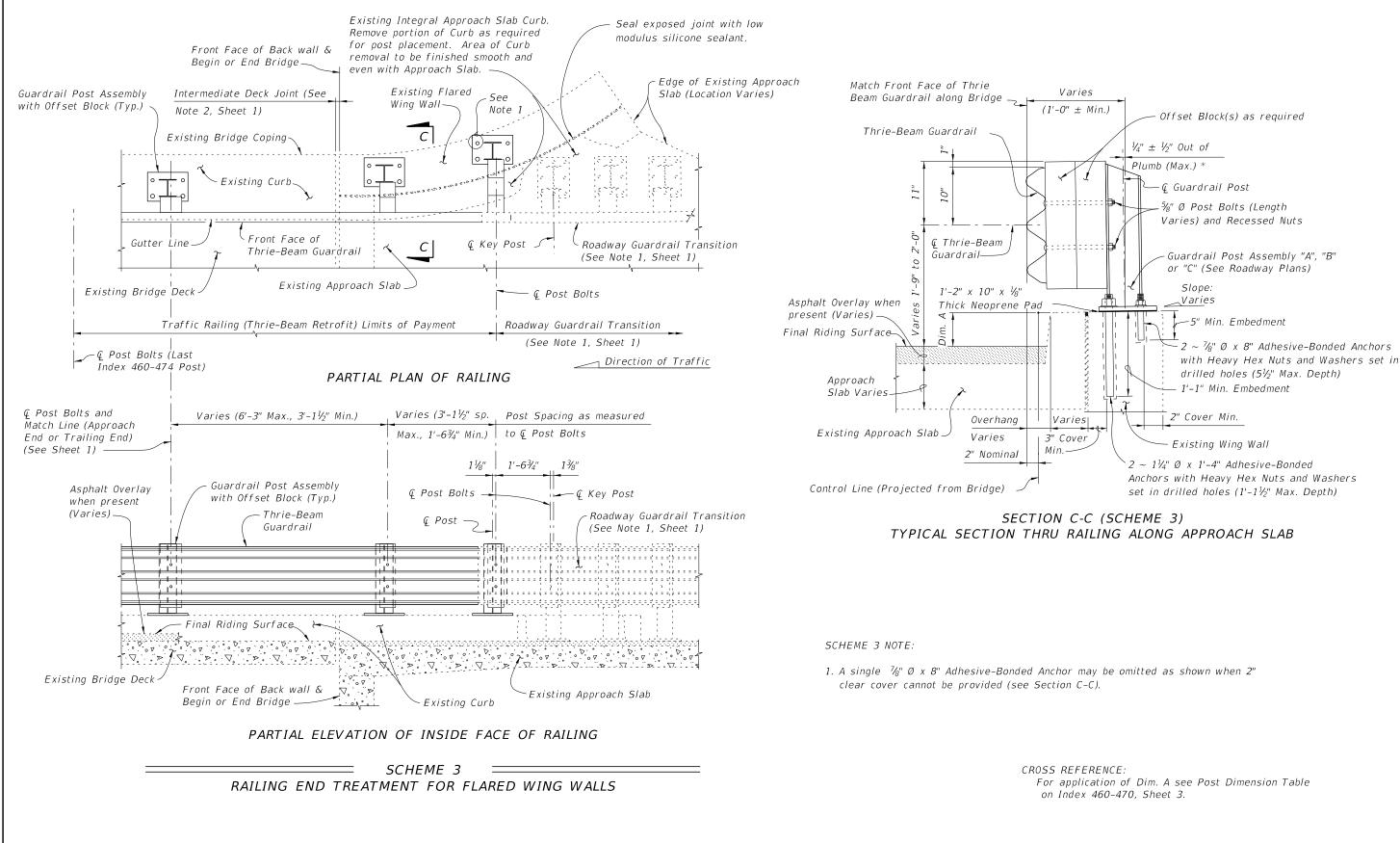
# CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3. For location of Section B-B see Sheet 3 For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

DESCRIPTION:

SHEET



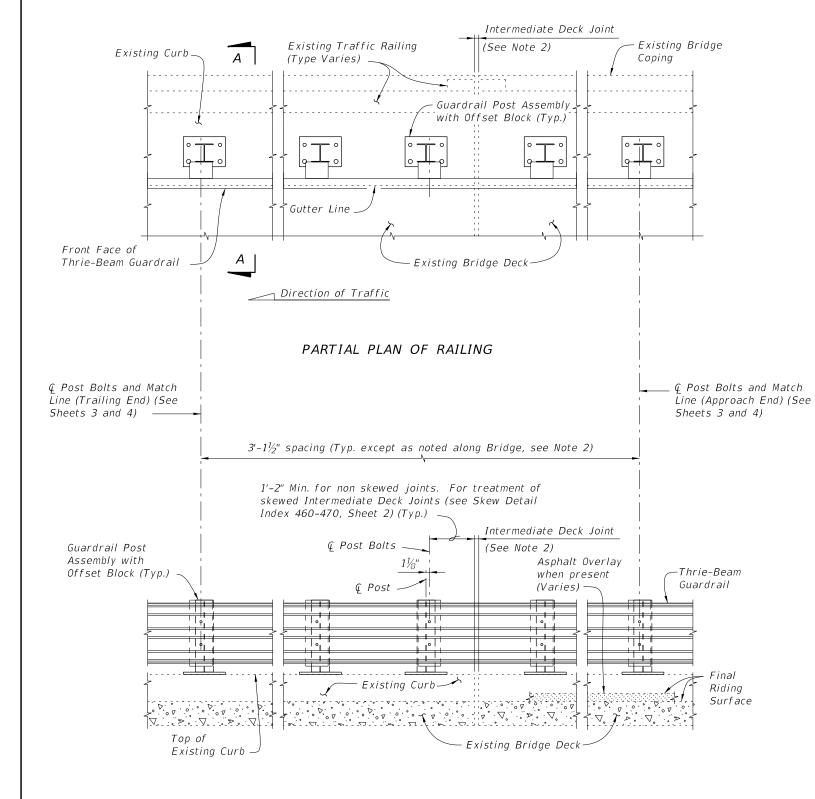


10/1/2021

LAST REVISION 07/01/09

DESCRIPTION:

FDOT



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

= TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

#### NOTES:

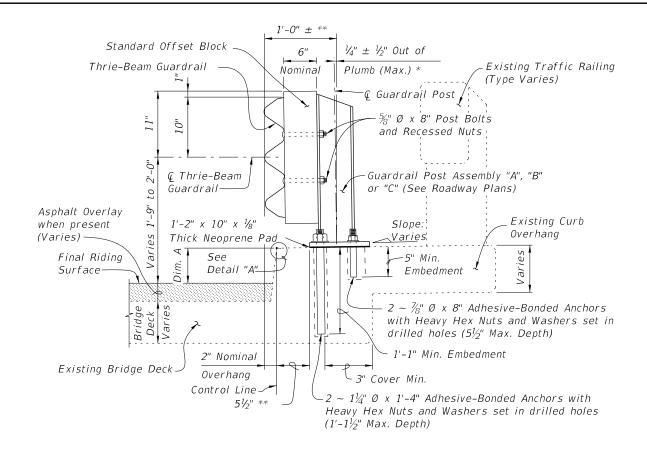
- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

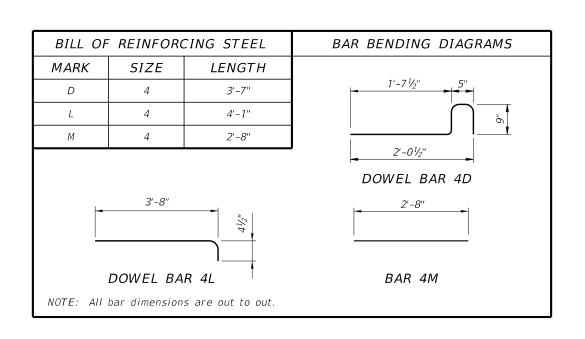
**REVISION** 01/01/08

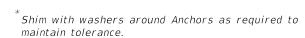
DESCRIPTION:

FDOT



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

when present

Surface

Approach

Slab Varies

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

Control Line (Schemes 5 & 6) -

Control Line (Projected from

Bridge) (Schemes 3 & 4) -

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required

Thrie-Beam

Guardrail-

♀ Thrie-Beam

1'-2" x 10" x 1/8"

Thick Neoprene Pad See

Detail "A'

-Existing

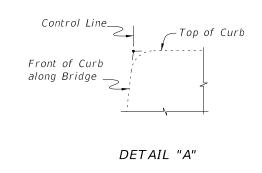
Approach

Slab

Varies 51/3" \*\*

Guardrail-

Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



Match shape of -Varies (Match existing curb curb height) Asphalt Overlay when present (Varies) 4M Existing 1'-41/2" Approach Slab Dowel Bars 4D (10" Embedment) Edge of Existing (See Note 2, Sheet 3) Approach Slab

VIEW C-C

### CROSS REFERENCES:

Varies \*\*

 $(1'-0'' \pm Min.)$ 

 $\frac{1}{4}$ "  $\pm \frac{1}{2}$ " Out of

Plumb (Max.) \*

-Ç Guardrail Post:

and Recessed Nuts

Slope:

Embedment

<u>\_\_\_Varie</u>s

\_5" Min.

└ 3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

%" Ø Post Bolts (length varies)

-Guardrail Post Assembly "A", "B"

Existing Curb Overhang

 $\sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5)

or  $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes  $(1'-1\frac{1}{2}'')$  or  $5\frac{1}{2}''$  Max.

drilled holes (5½" Max. Depth)

or "C" (See Roadway Plans)

Existing Wing Post

Existing Wing

(Type Varies)

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 01/01/08

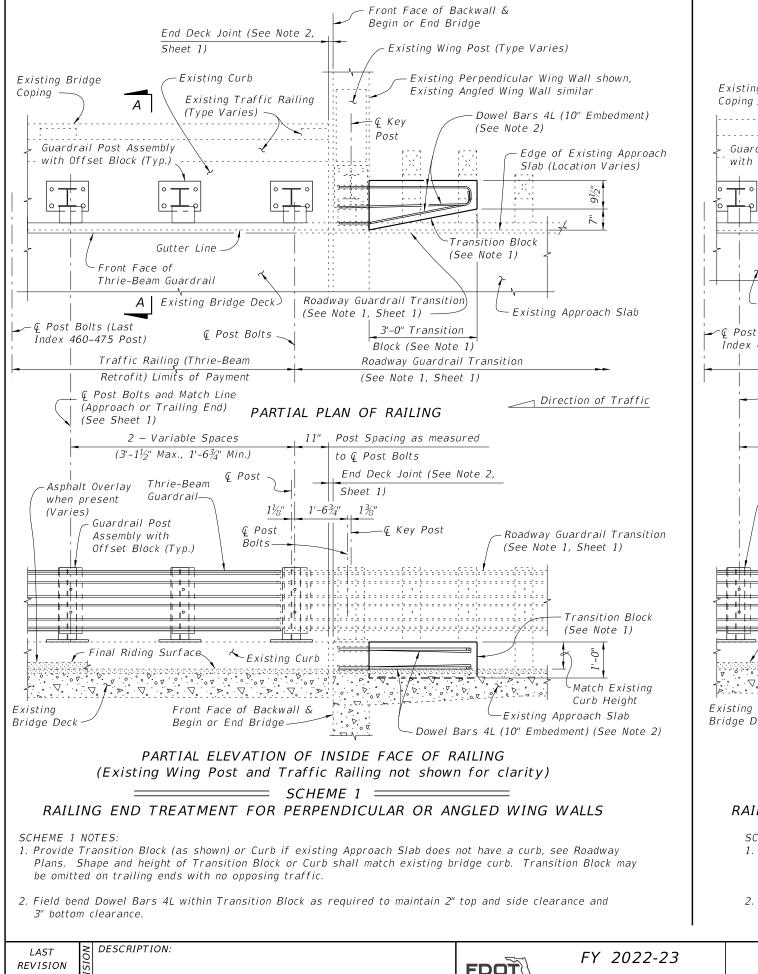
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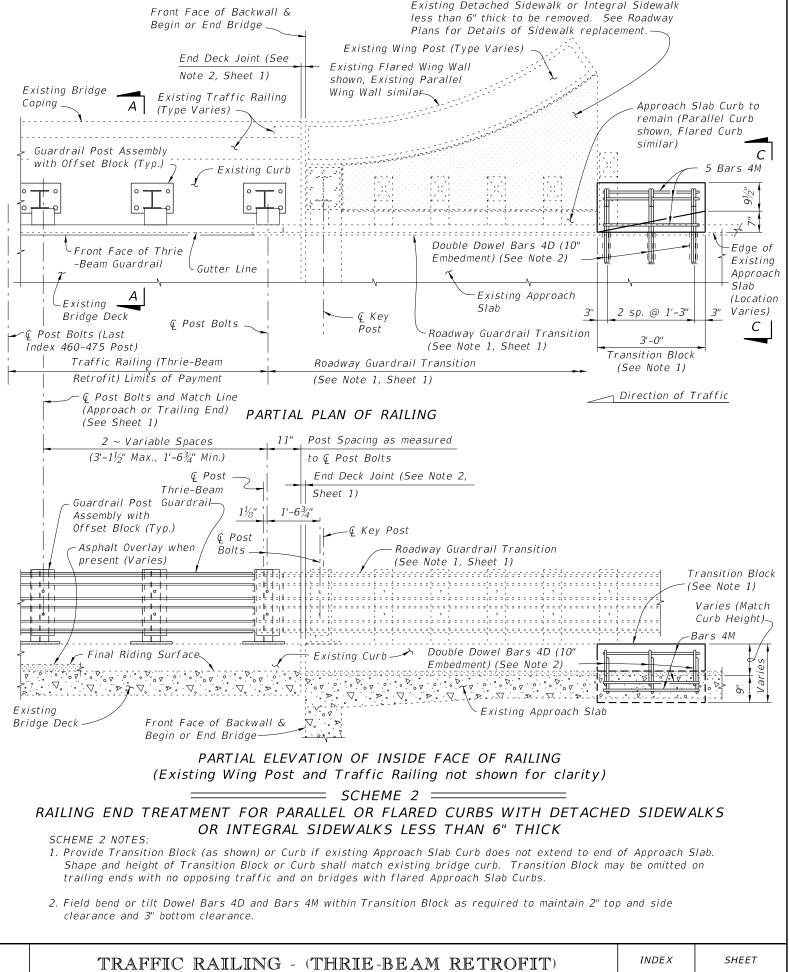
FY 2022-23 STANDARD PLANS TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

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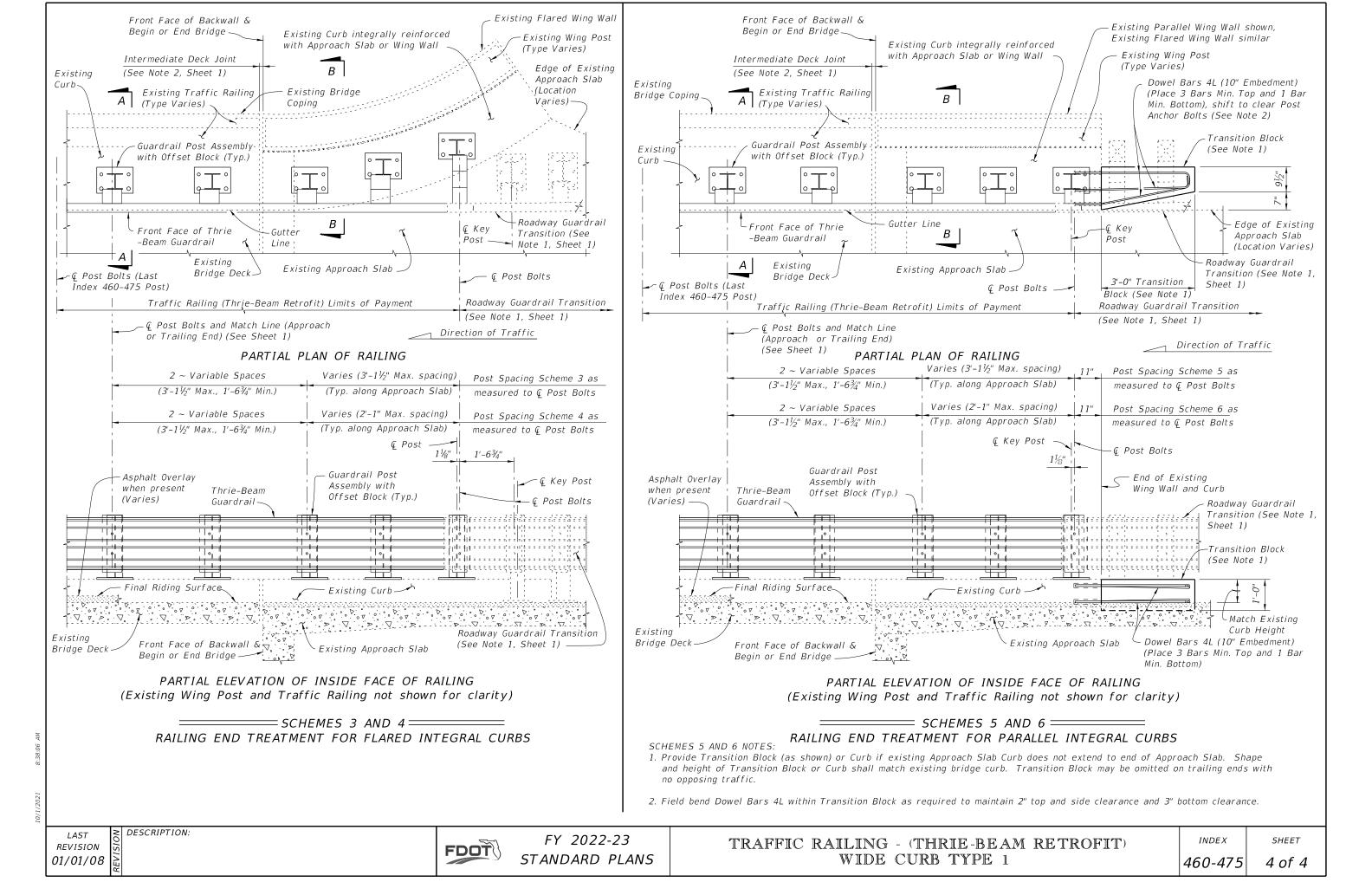


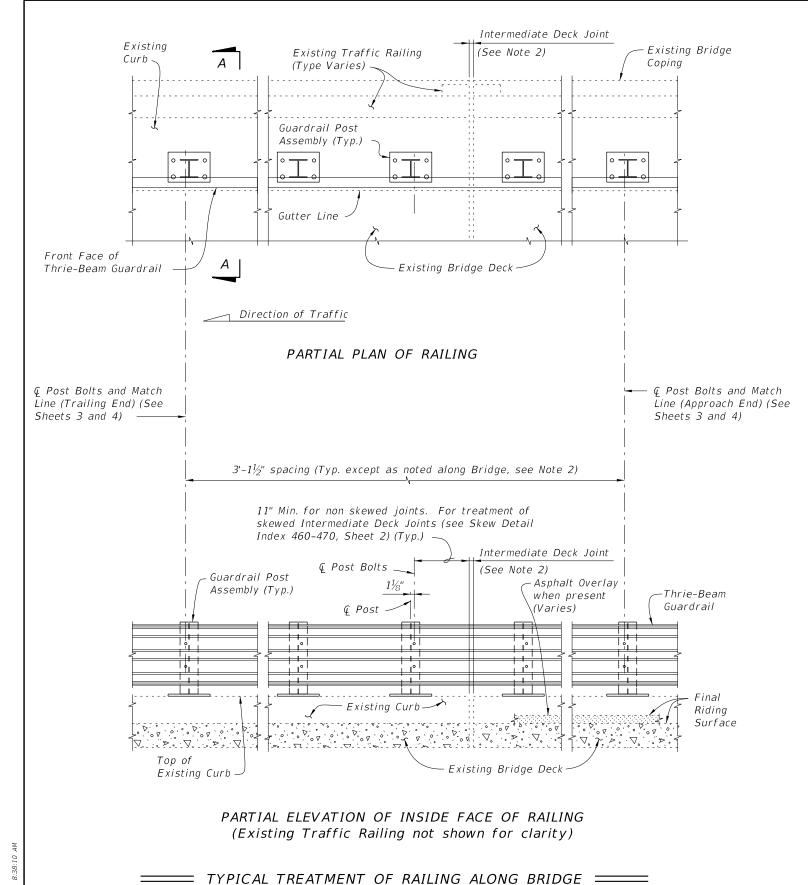


01/01/08

**FDOT** 

STANDARD PLANS





### NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

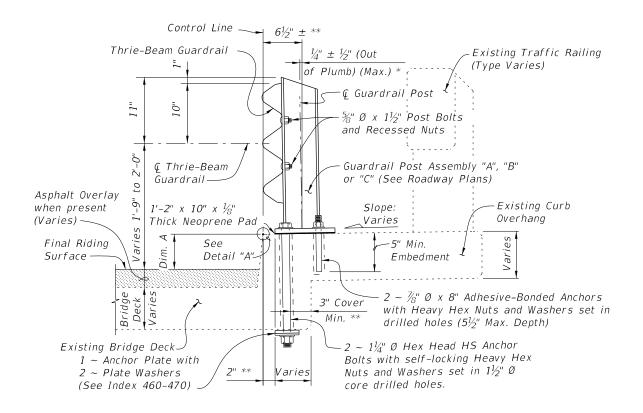
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

**REVISION** 01/01/08

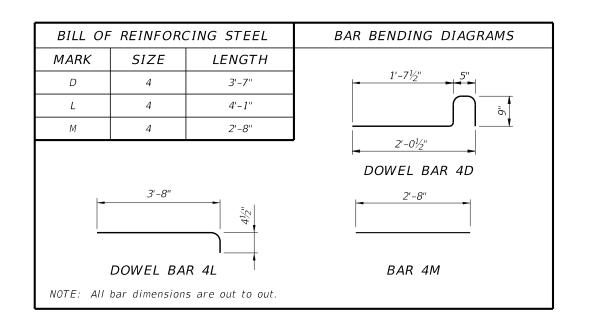
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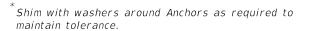
FDOT

FY 2022-23 STANDARD PLANS



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

Surface -

Approach

Slab Varies-

when present

(Varies) -

Thrie-Beam Guardrail along Bridge 🗲

Offset Block(s) as required

Thrie-Beam

Guardrail-

 ← Thrie-Beam

1'-2" x 10" x 1/3"

Thick Neoprene Pad

-Existing

Approach

Slab

Varies 51/2" \*\*

Guardrail

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - Nominal Overhang

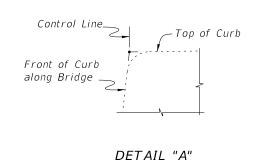
Control Line (Schemes 5 & 6)

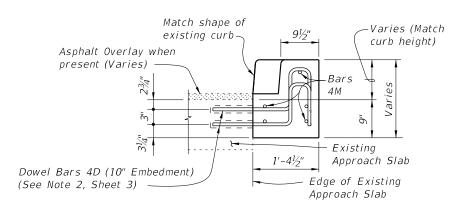
Control Line (Projected from

Bridge) (Schemes 3 & 4)

(Schemes 3 and 4 only)

Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

### CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

 $\frac{1}{4}$ "  $\pm \frac{1}{2}$ " Out of

• © Guardrail

%" Ø x 8" Post

Slope:

Depth respectively).

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Bolts and Recessed Nuts

or "C" (See Roadway Plans)

Guardrail Post Assembly "A", "B"

Varies¦ Embedment

Existing Curb Overhang

\_\_ 5" Min.

drilled holes (5½" Max. Depth)

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5) or  $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes  $(1'-1\frac{1}{2}'')$  or  $5\frac{1}{2}''$  Max.

Plumb (Max.) \*

Existing Wing Post

Existing Wing

Wall

%" Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

(Type Varies)

For location of Section C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

**REVISION** 07/01/08

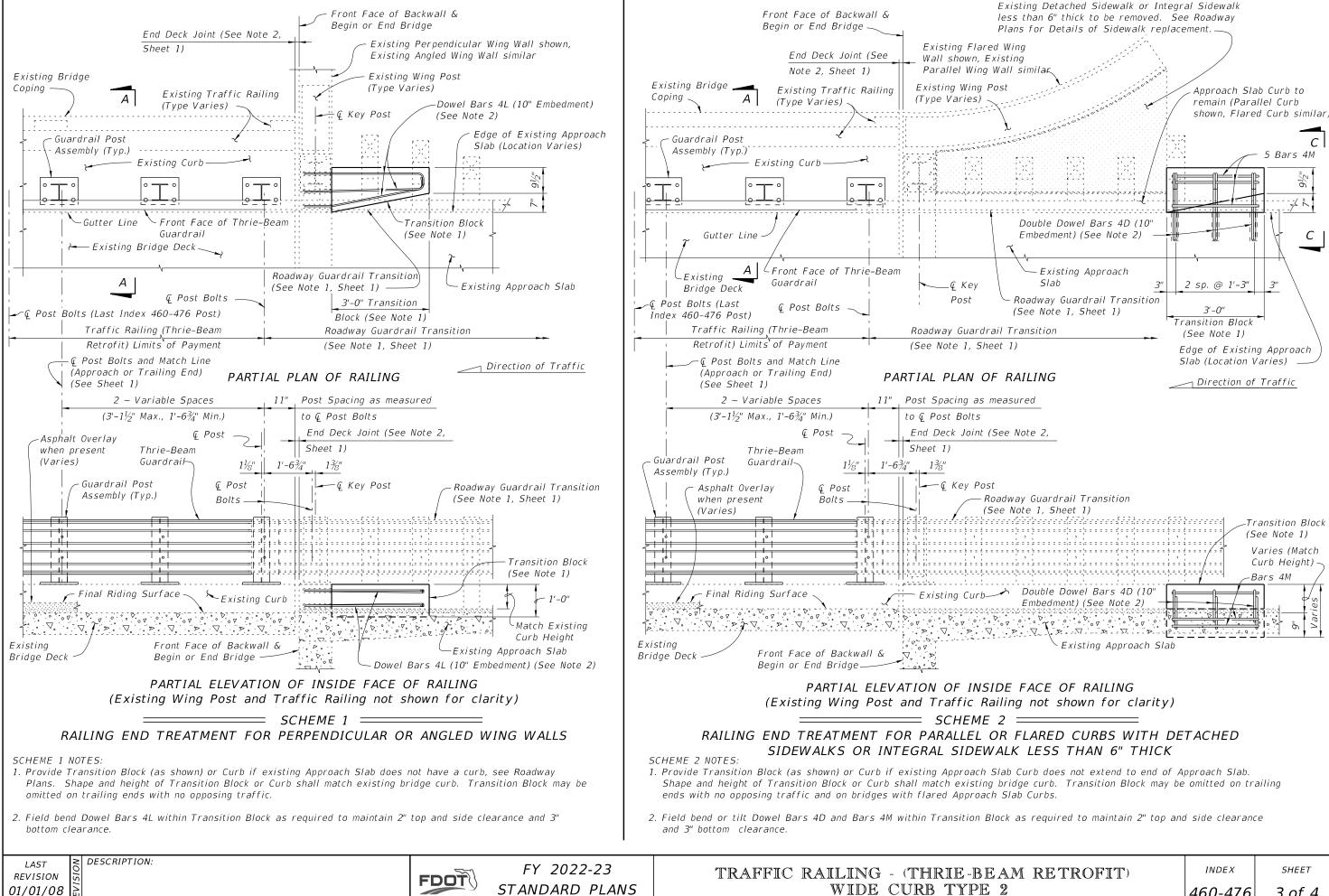
DESCRIPTION:

FDOT

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SHEET



С

9½"

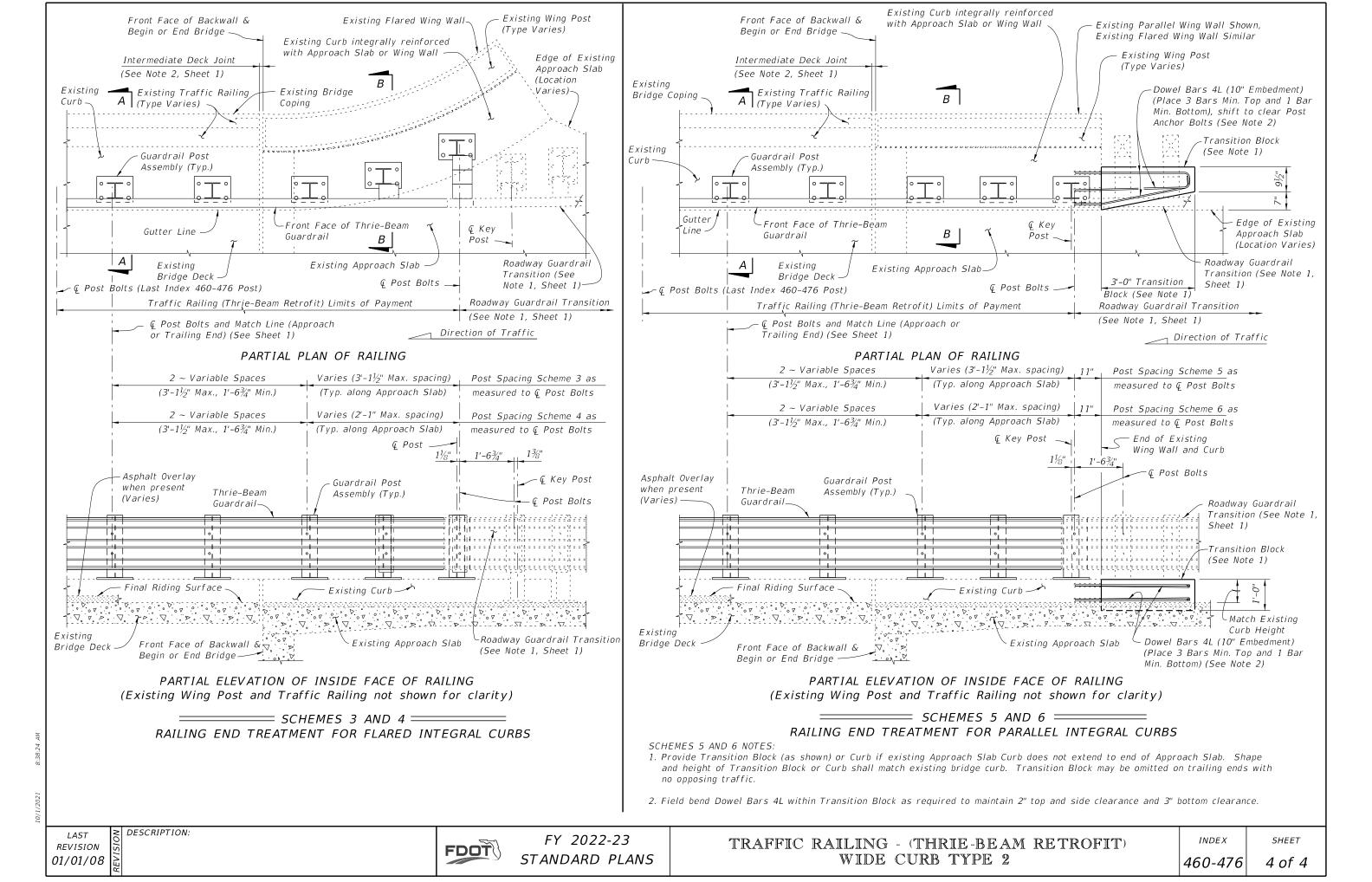
5 Bars 4M

-Transition Block

Curb Height) -

(See Note 1) Varies (Match

-Bars 4M



CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be  $\frac{3}{4}$ " by  $2\frac{1}{2}$ " slotted holes.

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

COATINGS: All Nuts, Bolts, Anchors, and Washers shall be hot-dip galvanized in accordance with the Specifications.

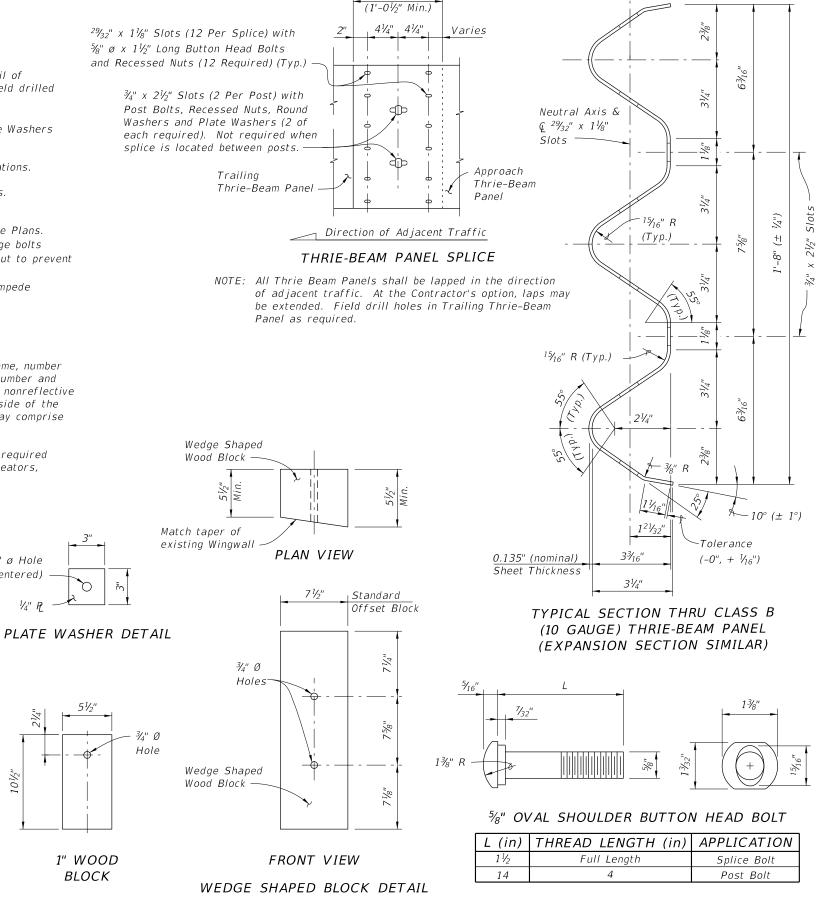
BRIDGES ON CURVED ALIGNMENTS: The details presented herein are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$ " slots in thrie-beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten bolts in 3¾" slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

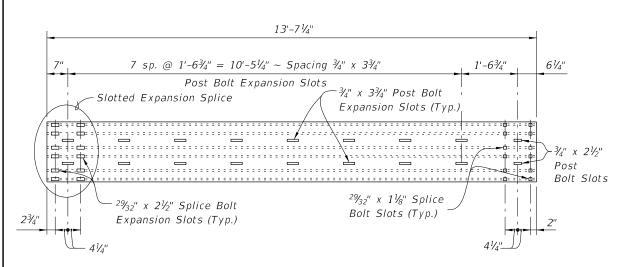
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered ( $\pm \frac{1}{4}$ ").

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie-Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent quardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

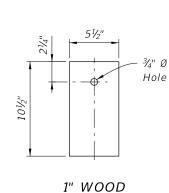
PAYMENT: Payment will be made under Thrie-Beam Panel Retrofit which shall include all materials and labor required to fabricate and install the retrofit railing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators, where required, will not be paid for directly but shall be considered incidental work.



Lap



THRIE-BEAM EXPANSION SECTION



¾" ø Hole

(centered)

1/4" P

**REVISION** 07/01/14

**FDOT** 

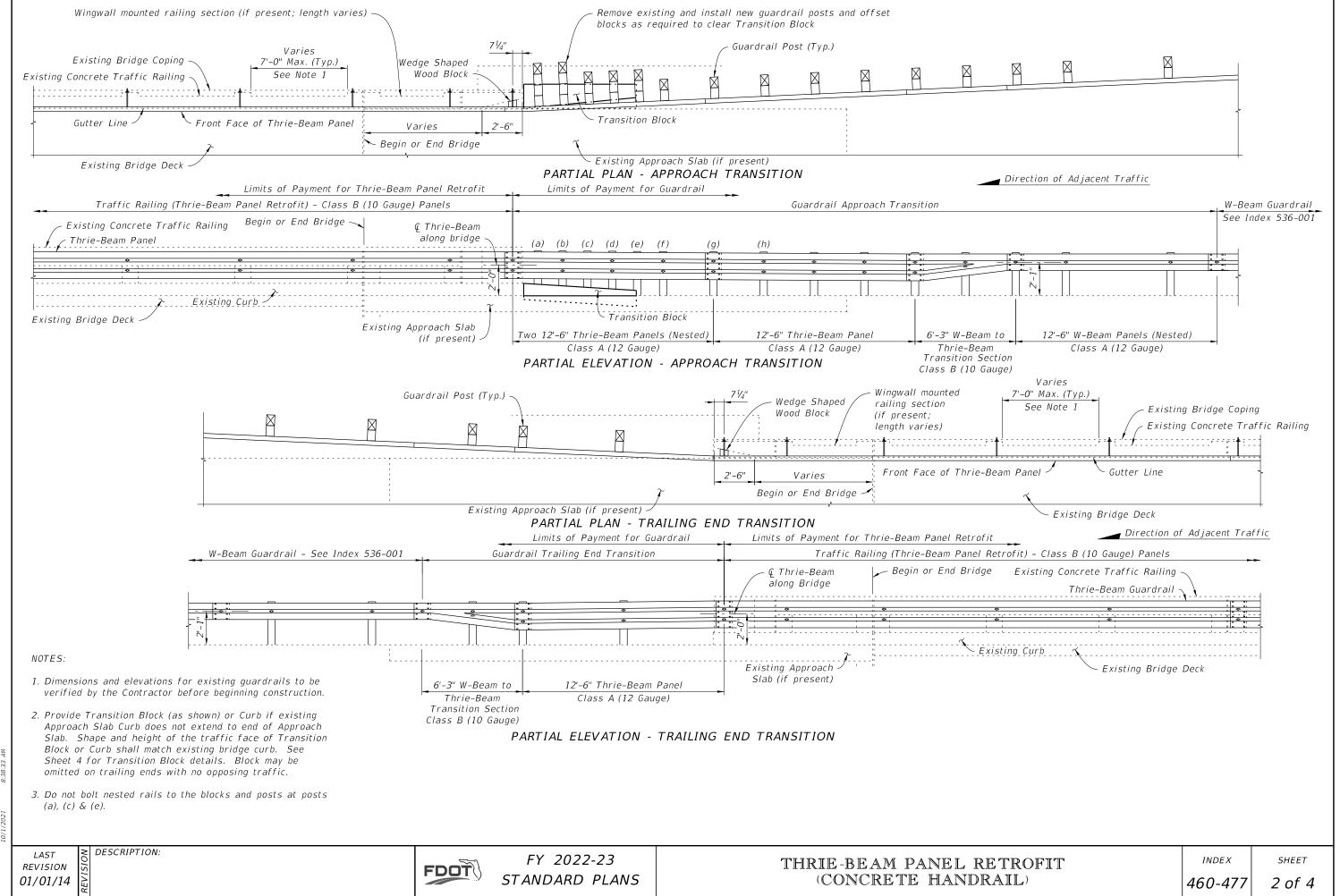
FY 2022-23 STANDARD PLANS

THRIE-BEAM PANEL RETROFIT

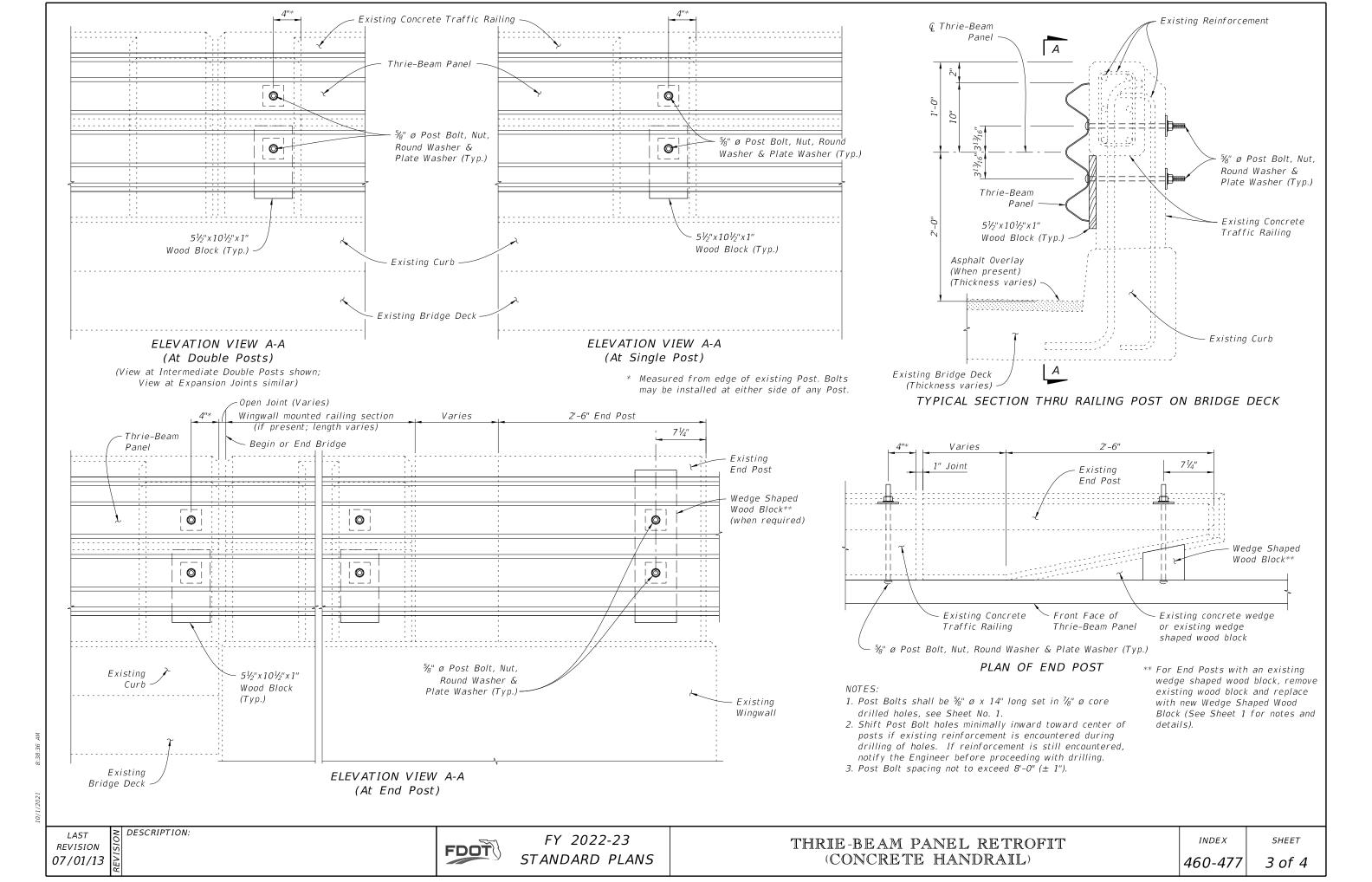
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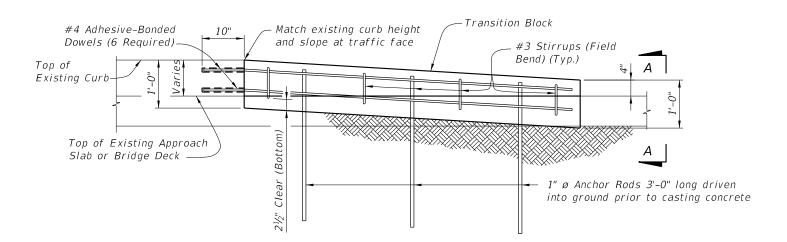
DESCRIPTION:



10/1/2021

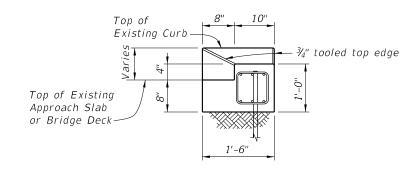


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

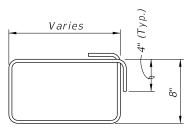


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK		
ITEM	UNIT	QUANTITY
Concrete Class II (Bridge Deck)	CY	0.4
Reinforcing Steel	LB	61
Guardrail (Reset)	LF	12.5



END VIEW A-A



#3 STIRRUP (FIELD BEND)

### NOTES:

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

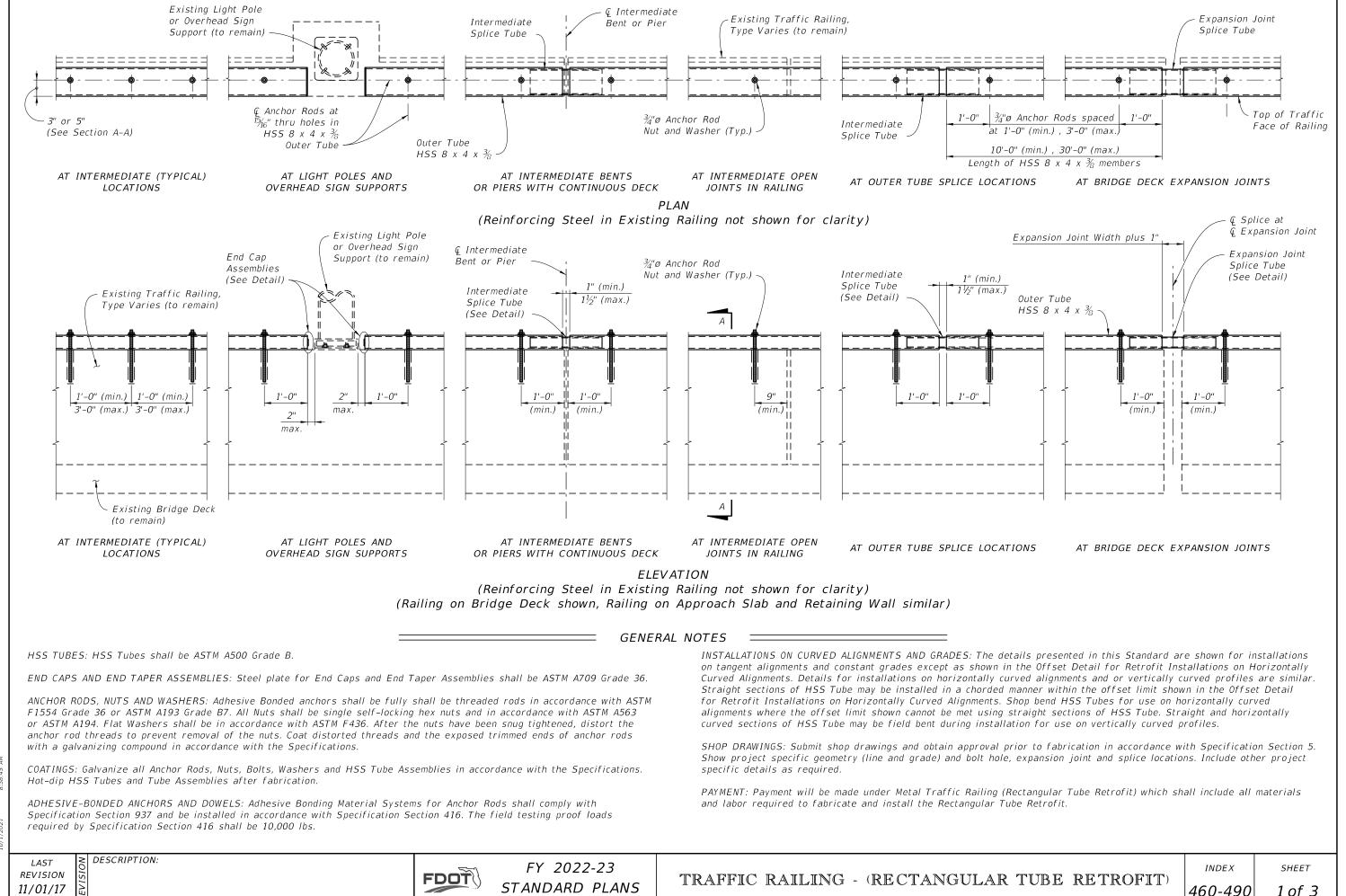
ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 (Type HV) and be installed in accordance with Specification Section 416.

Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

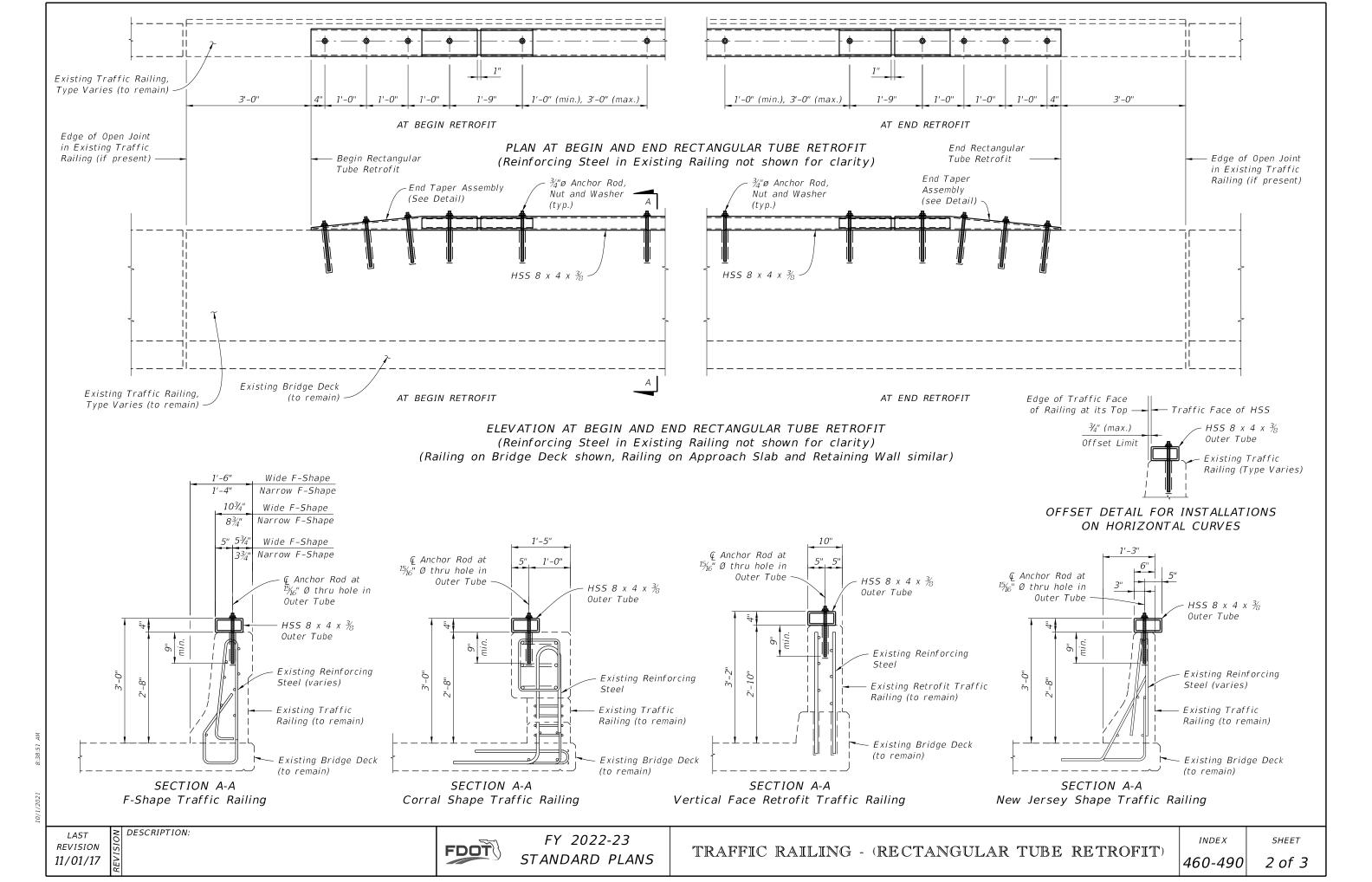
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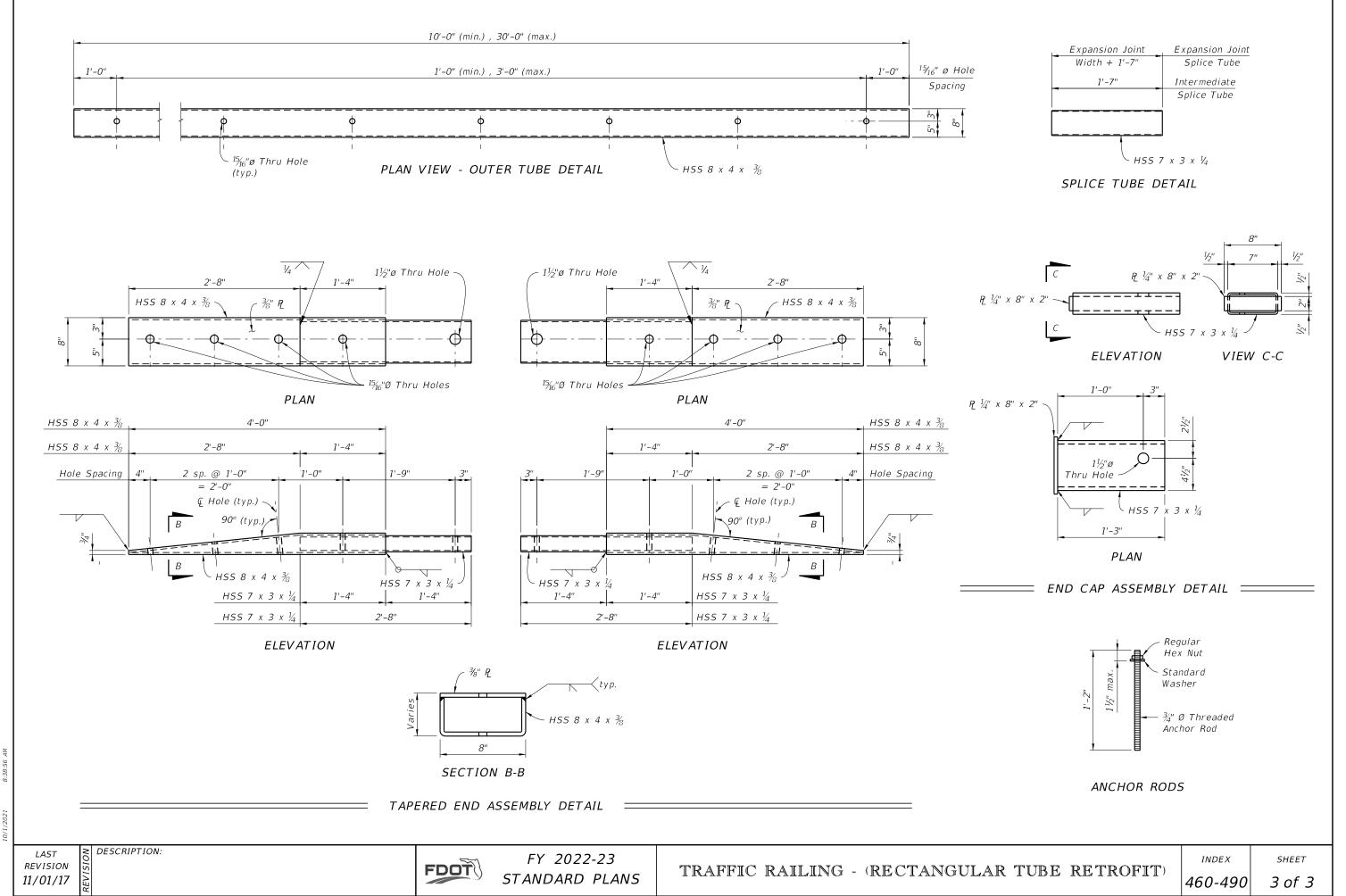
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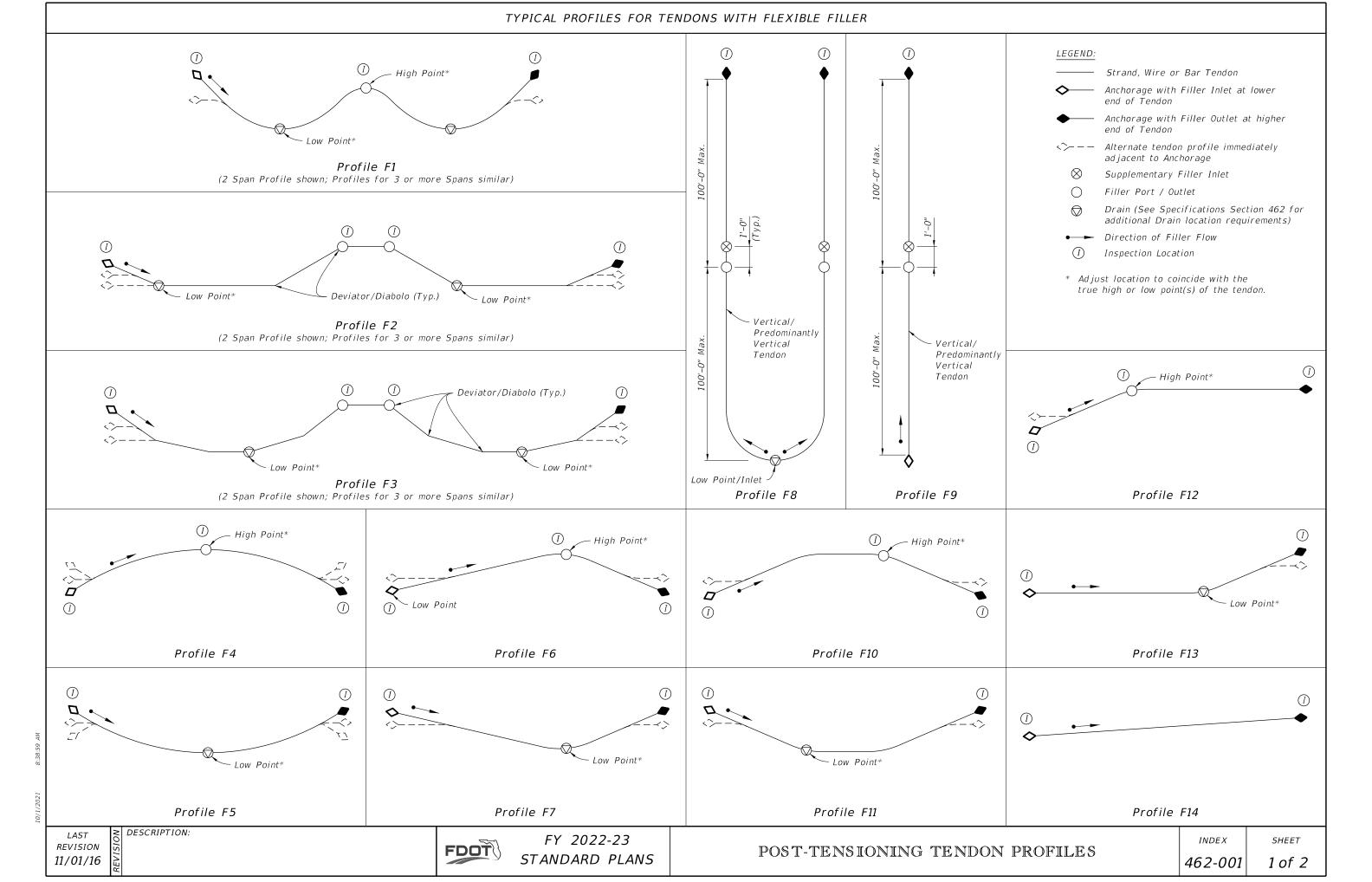


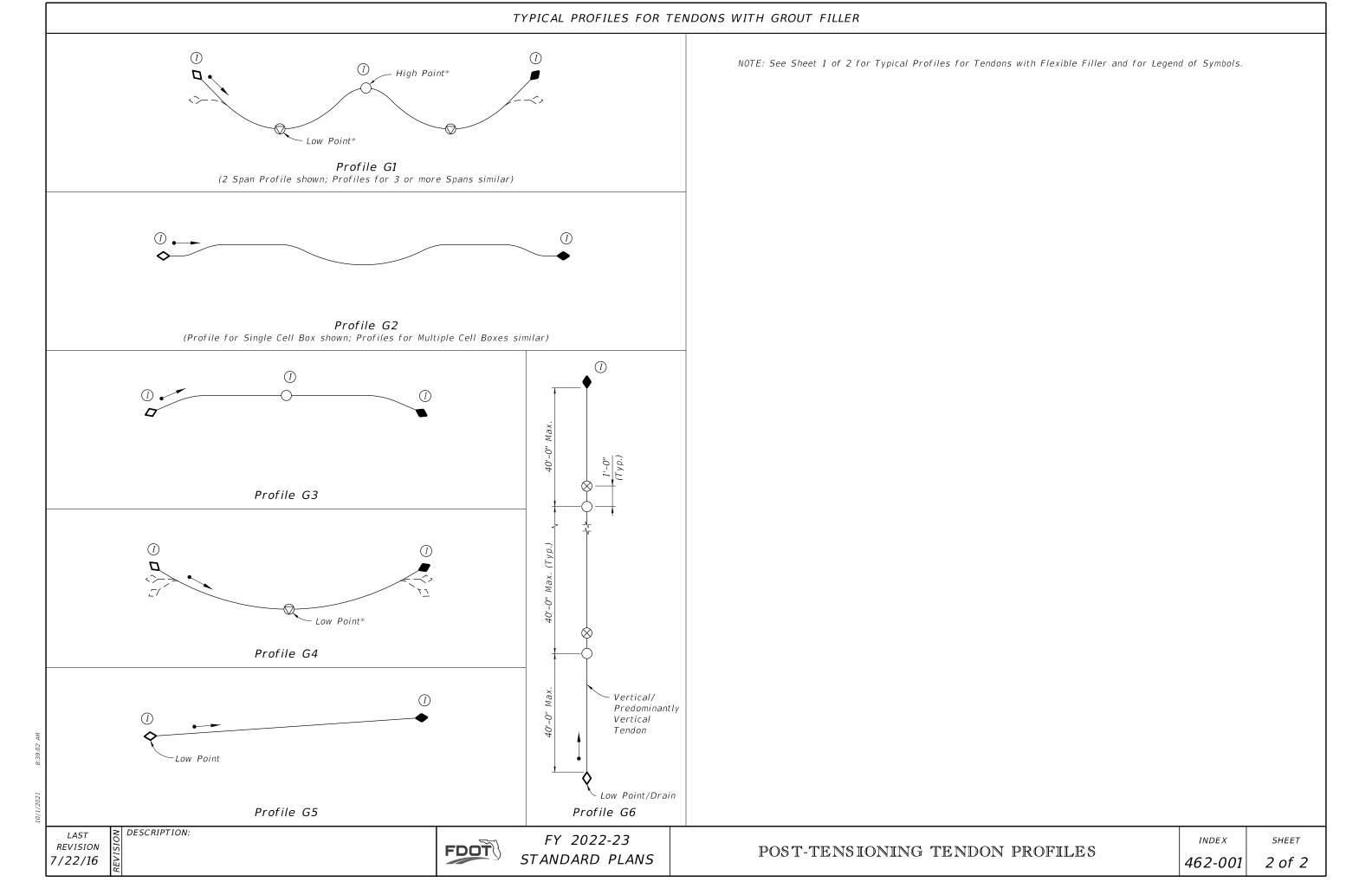
11/01/17

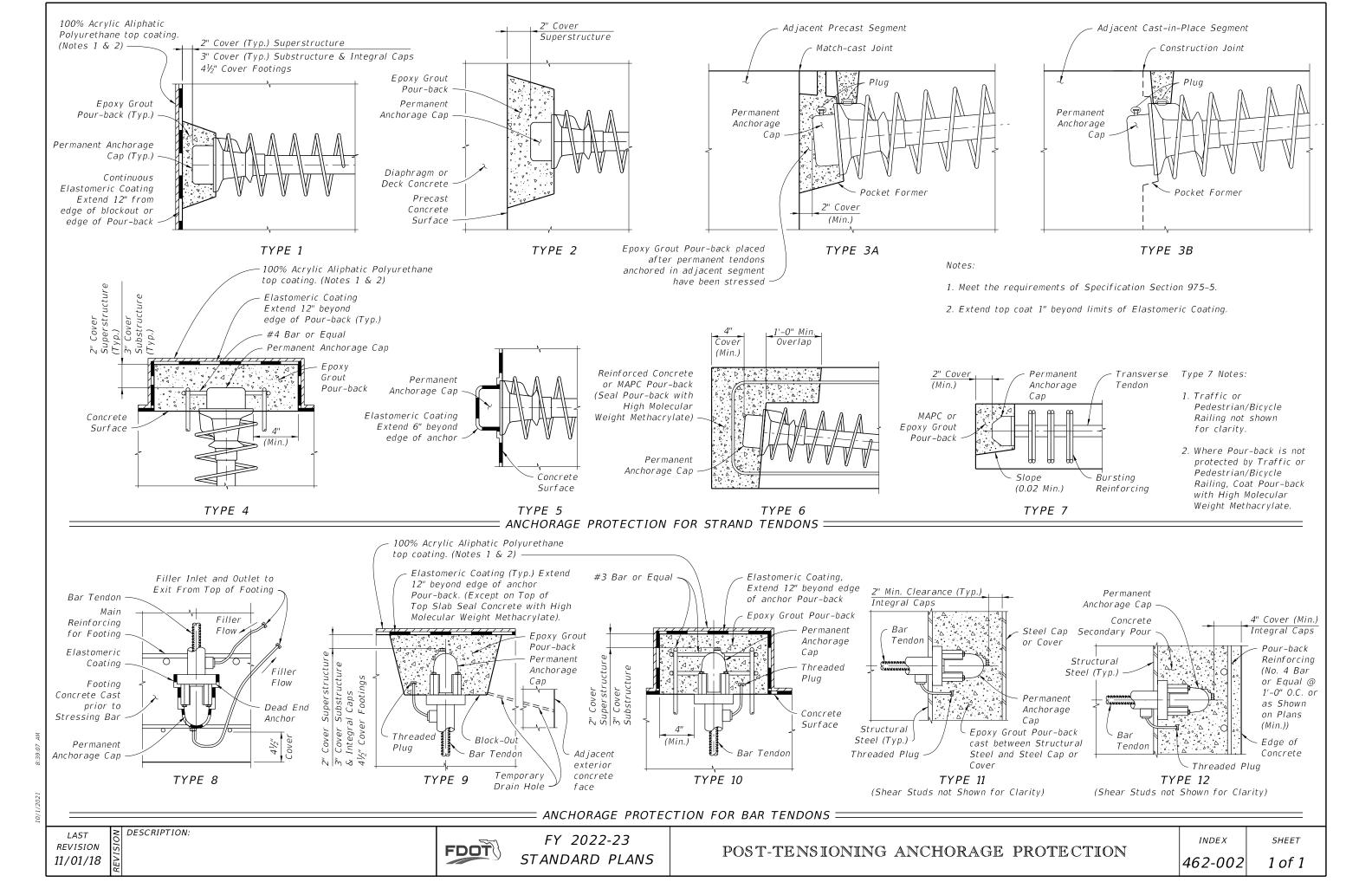
STANDARD PLANS

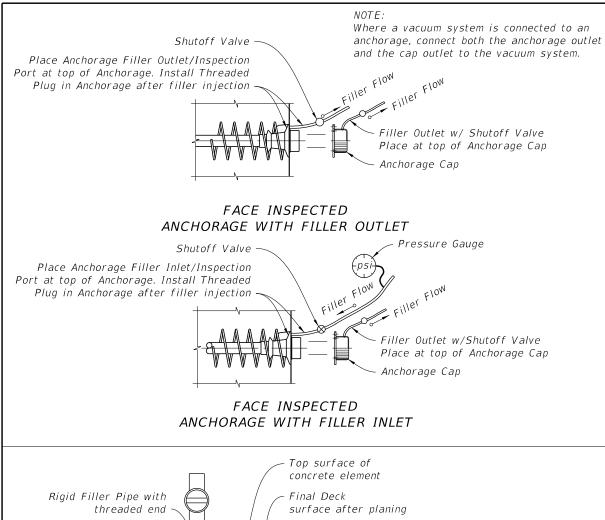


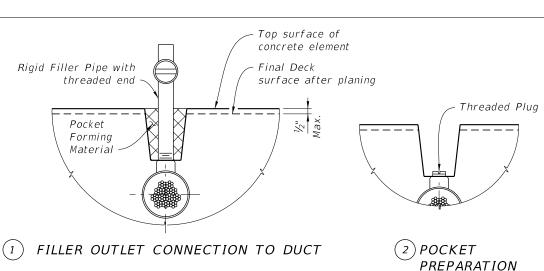


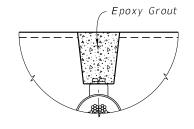












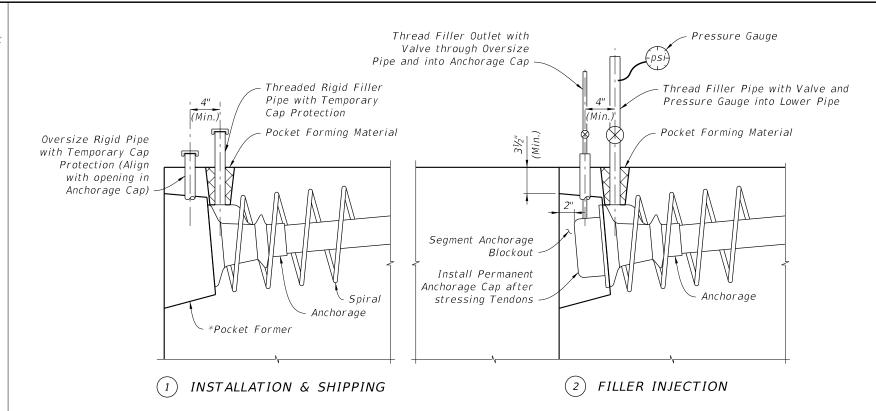
### PROCEDURE:

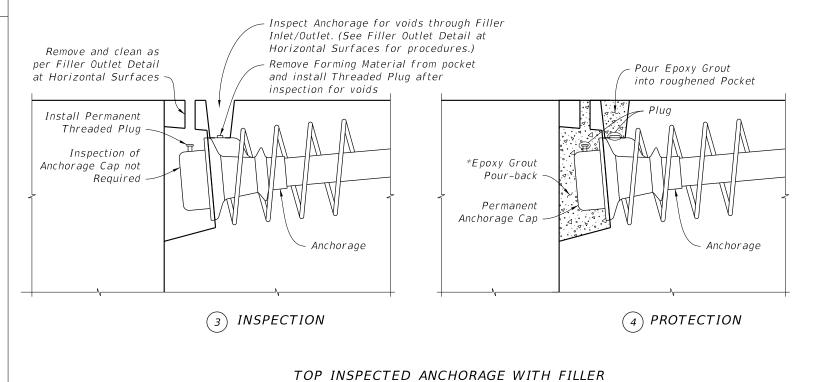
- 1. After filler injection is completed, Remove Pocket Forming Material and Rigid Filler Pipe.
- 2. Inspect Tendon for voids as necessary.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Clean threads and rethread as required.
- 5. Install Threaded Plug into Outlet to form a tight fit.
- 6. Clean and roughen sides of pocket.
- 7. Fill Pocket with Epoxy Grout.

(3) FILLING POCKET

DESCRIPTION:

= FILLER OUTLET DETAIL AT HORIZONTAL SURFACES =





### NOTES:

- 1. Holes used for the Inspection and Filler Inlets/Outlets may be formed using tapered pipes or mandrels.
- 2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.
- \* Round ( ) Pocket Former Gravity fed placement of epoxy grout acceptable Modified Square Pocket Former - Gravity fed placement of epoxy grout acceptable Square Pocket Former - Vacuum epoxy grouting required

**REVISION** 11/01/18

FDOT

FY 2022-23

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

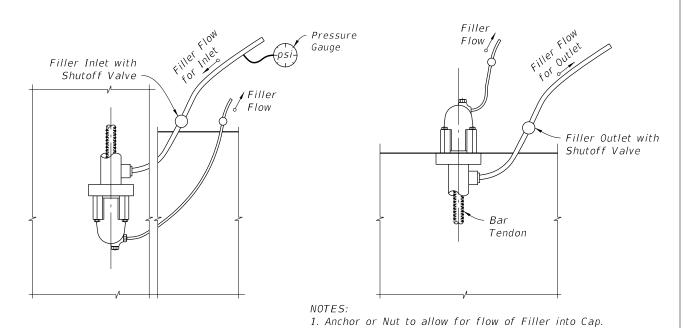
INLET INSTALLATION, FILLER INJECTION.

INSPECTION & PROTECTION

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### INLET END

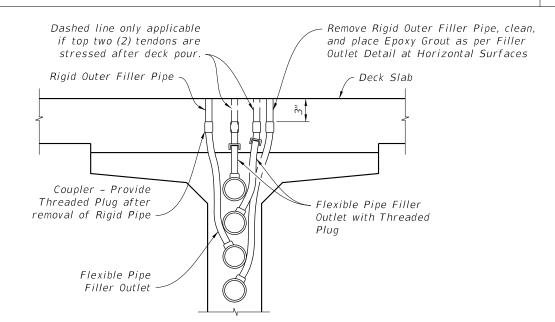
(EMBEDDED ANCHORAGE SHOWN; ANCHORAGE AT CONCRETE SURFACE SIMILAR)

2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.

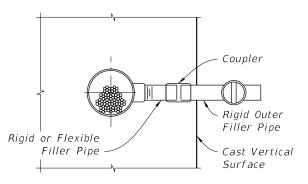
### **OUTLET END**

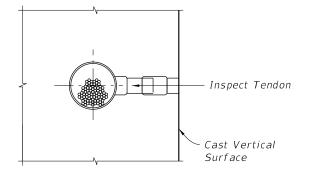
FILLER INLET AND OUTLET DETAILS FOR BAR TENDONS

(VERTICALLY ORIENTED TENDON SHOWN; HORIZONTALLY ORIENTED TENDON SIMILAR)



TENDONS AT HIGH POINTS AND 3' FROM HIGH POINTS (FILLER OUTLET)





(1) FILLER OUTLET CONNECTION TO TENDON

(3) FILLING POCKET

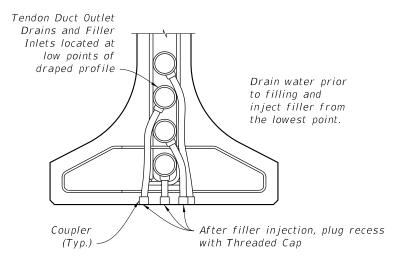
## Epoxy Grout Threaded Plug Cast Vertical Surface

(2) POCKET PREPARATION

### PROCEDURE:

- 1. Remove Rigid Filler Pipe or drill Grout in flexible pipe.
- 2. Inspect tendon for voids.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Install Threaded Plug into Outlet to form a tight fit.
- 5. Over-ream hole ( $\frac{1}{4}$ " Ø over-ream). Clean and roughen sides.
- 6. Fill pocket with epoxy grout.

FILLER OUTLET DETAIL AT VERTICAL SURFACES



TENDONS AT LOW POINTS (FILLER INLET / DRAIN)

== FILLER INLET AND OUTLET DETAILS FOR I-GIRDERS ======

DETAILS FOR C.I.P. BOXES WITH INTERNAL TENDONS SIMILAR, WEB REINFORCING NOT SHOWN FOR CLARITY.

**REVISION** 11/01/16

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

AND TENDON FILLING DETAILS

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SHEET

POST-TENSIONING ANCHORAGE

- U.S. COAST GUARD NOTIFICATION: Notify the local office of the U.S. Coast Guard at least 30 days prior to beginning of construction of the Fender System.
- 14" SQUARE PRESTRESSED CONCRETE PILES Provide 14" Square Prestressed Concrete Piles of sufficient length to achieve a minimum embedment of 20' into soil having a blow count greater than or equal to 6 ( $N \ge 6$ ). Pile splices and build-ups are not permitted. Use only 14" Square Prestressed Concrete Piles with  $8 \frac{1}{2}$ " diameter Low Relaxation Strands fabricated in accordance with Index 455–014.
- PLASTIC LUMBER AND STRUCTURAL COMPOSITE LUMBER WALES: Provide only Plastic Lumber (Thermoplastic Structural Shapes) and Structural Composite Lumber (Reinforced Thermoplastic Structural Shapes) Wales in accordance with Specification Section 973. Wales shall be continuous and spliced only at locations shown on the plans.
- PLASTIC LUMBER DECKING FOR CATWALKS: Provide Plastic Lumber decking for catwalks when called for in the Plans in accordance with Specification Section 973.

Install Plastic Lumber Decking according to manufacturer's recommendations using stainless steel  $\#10 \times 3$ " (minimum) deck screws.

FIBERGLASS OPEN GRATING FOR CATWALKS: Provide Fiberglass Open Grating for catwalks when called for in the Plans. Fiberglass Open Grating shall be a heavy duty design suitable for exterior installations. Maximum gap opening on the walkway surface shall be  $1\frac{1}{2}$ ". Design live loads and deflections shall be a 50 psf uniformly distributed load with a maximum deflection of  $\frac{3}{8}$ " or L/120 at the center of a simple span and a concentrated load of 250 pounds with a maximum deflection of  $\frac{1}{4}$ " at the center of a simple span. Color of Fiberglass Open Grating shall be gray or black.

Install Fiberglass Open Grating according to manufacturer's recommendations using stainless steel hardware, screws, bolts, nuts and washers. Attach Fiberglass Open Grating to Wales and Deck Supports at a 2'-0" maximum spacing so as to resist pedestrian live loads and uplift forces from wind, buoyancy and wave action.

- CLEARANCE GAUGE AND LIGHT: Clearance Gauge to be furnished and installed by the Contractor. Clearance Gauge width and numeral height is dependent on visibility distance. The required visibility distance shall be determined by the United States Coast Guard District Commander. Provide and install Clearance Gauge Light in accordance with Specification Section 510 and Index 510-001.
- NAVIGATION LIGHTS: Provide and install Navigation Lights in accordance with Specification Section 510, Index 510-001 and/or project specific details. Provide and maintain Temporary Navigation Lights during construction until permanent Navigation Lights are operational.
- BOLTS, THREADED BARS, NUTS, SCREWS AND WASHERS: Furnish stainless steel Bolts in accordance with ASTM F593 Type 316. Furnish stainless steel Threaded Bars in accordance with ASTM A193 Grade B8M. Furnish stainless steel Nuts in accordance with ASTM F594 Type 316. Furnish stainless steel Screws in accordance with ASTM F593 Type 305. Furnish stainless steel Washers compatible with Bolts, Threaded Rods and Nuts under heads and nuts. Torque Nuts on 1" diameter Bolts and Threaded Bars to 150 lb-ft. Keep threads on Bolts, Threaded Bars and Nuts free from dirt, coarse grime and sand to prevent galling and seizing during tightening.

SPLICE PLATES: Furnish Splice Plates in accordance with ASTM A240 Type 316.

WIRE ROPE: Provide wire rope meeting one of the following requirements:

- 1.  $\frac{1}{2}$ " diameter 6x19, 6x25 or 6x37 class IWRC Type 316 stainless steel wire rope with a minimum breaking strength of 18.000 lbs.
- 2. ½" diameter 6x19 galvanized wire rope with ultraviolet ray resistant polypropylene impregnation having an outside diameter of 5/8" with a minimum breaking strength of 22,000 lbs. Protect all ends with heat shrinkable end caps compatible with the rope's polypropylene that provide an effective water-tight seal.

FENDER SYSTEM ENERGY CAPACITY: Energy Capacity = 38 ft-k

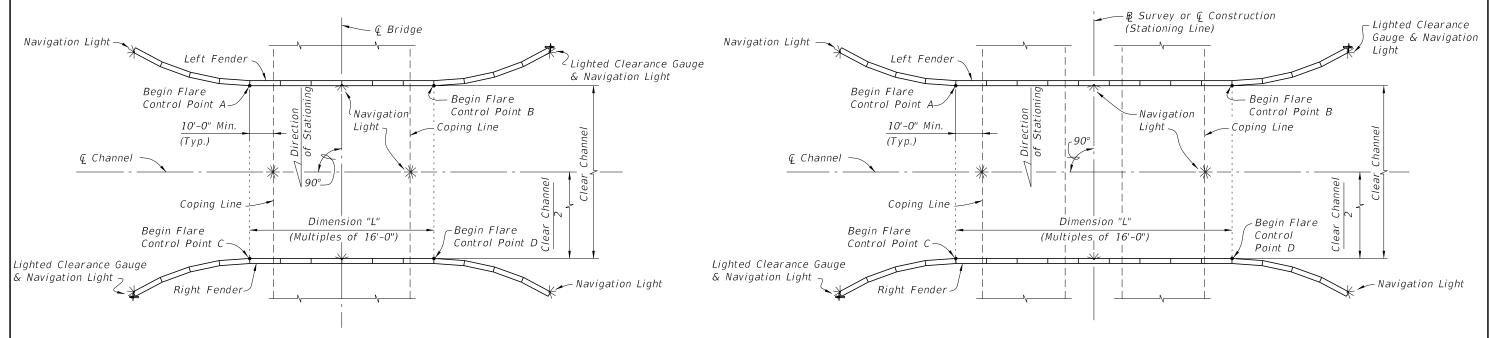
GENERAL NOTES

10/1/2021

LAST REVISION 07/01/14

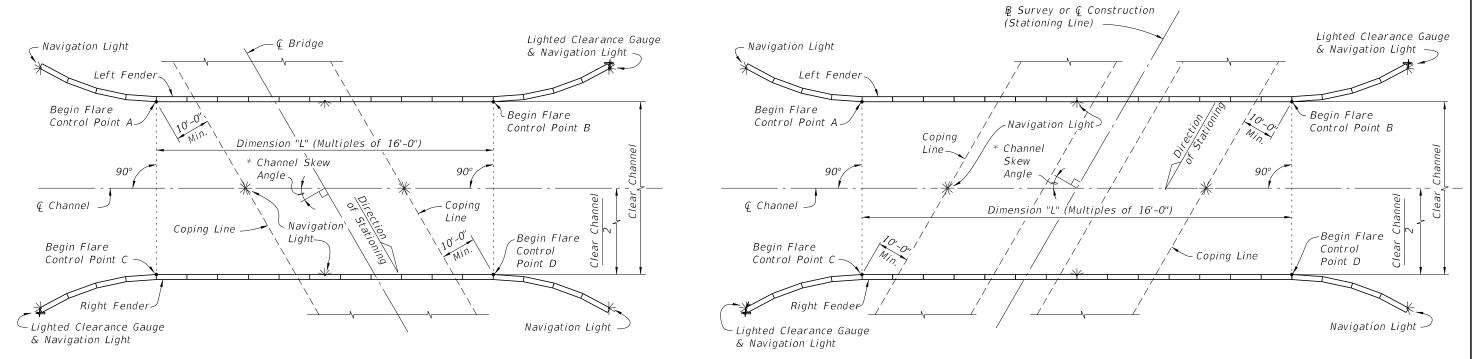
DESCRIPTION:

FDOT



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH NONSKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH NONSKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH SKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH SKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)

\* See Structures Plans, Plan and Elevation and Foundation Layout Sheets for magnitude and orientation of Channel Skew Angle.

CROSS REFERENCES:

For Stations and Offsets of referenced Control Points A, B, C and D, Dimension "L" and Clear Channel Width see Fender System Table of Variables in Structures Plans.

For Navigation Light Details see Design Standards Index 510-001.

LAYOUT GEOMETRY

**REVISION** 07/01/11

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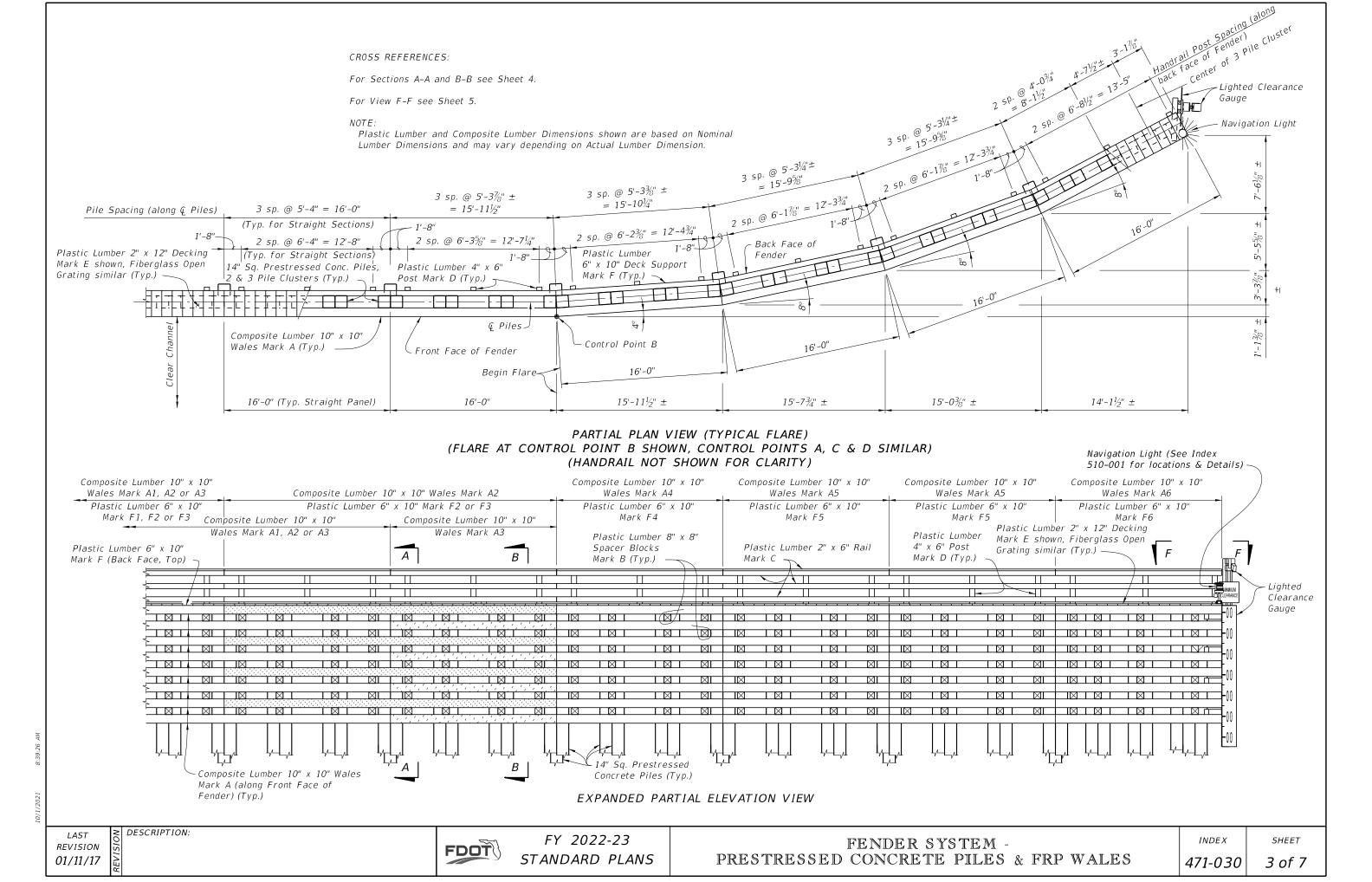
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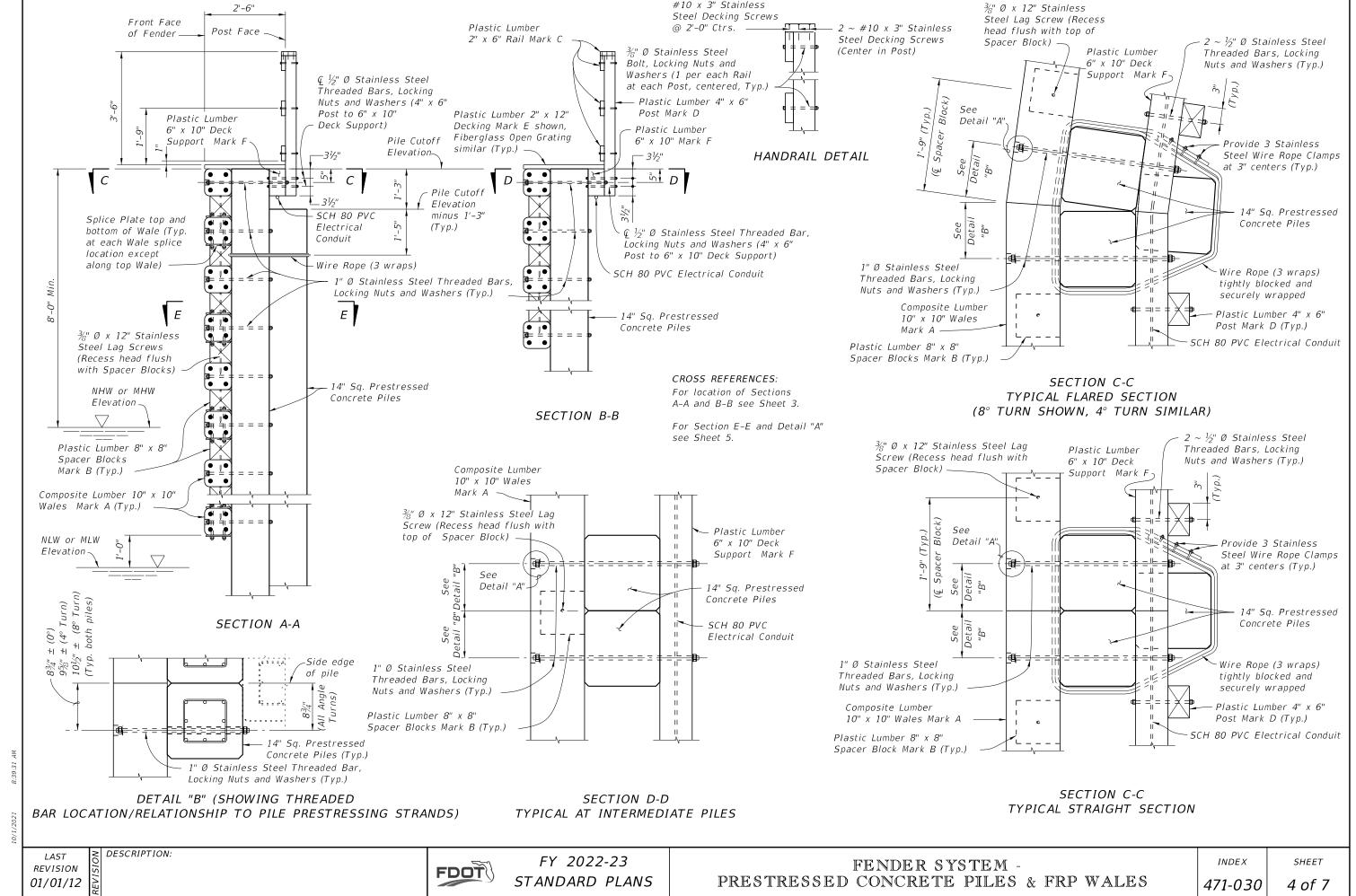
FY 2022-23 STANDARD PLANS

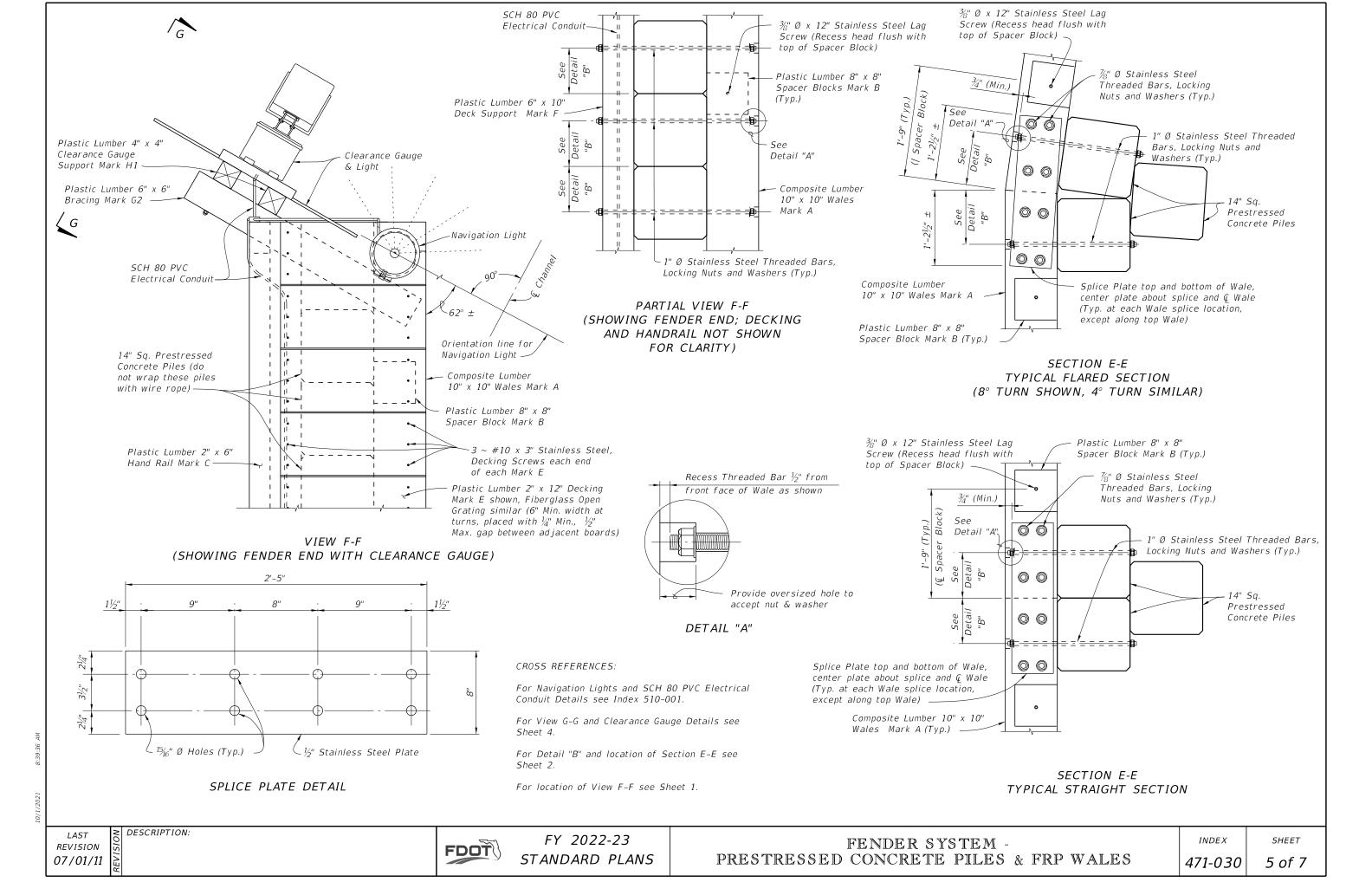
FENDER SYSTEM -PRESTRESSED CONCRETE PILES & FRP WALES

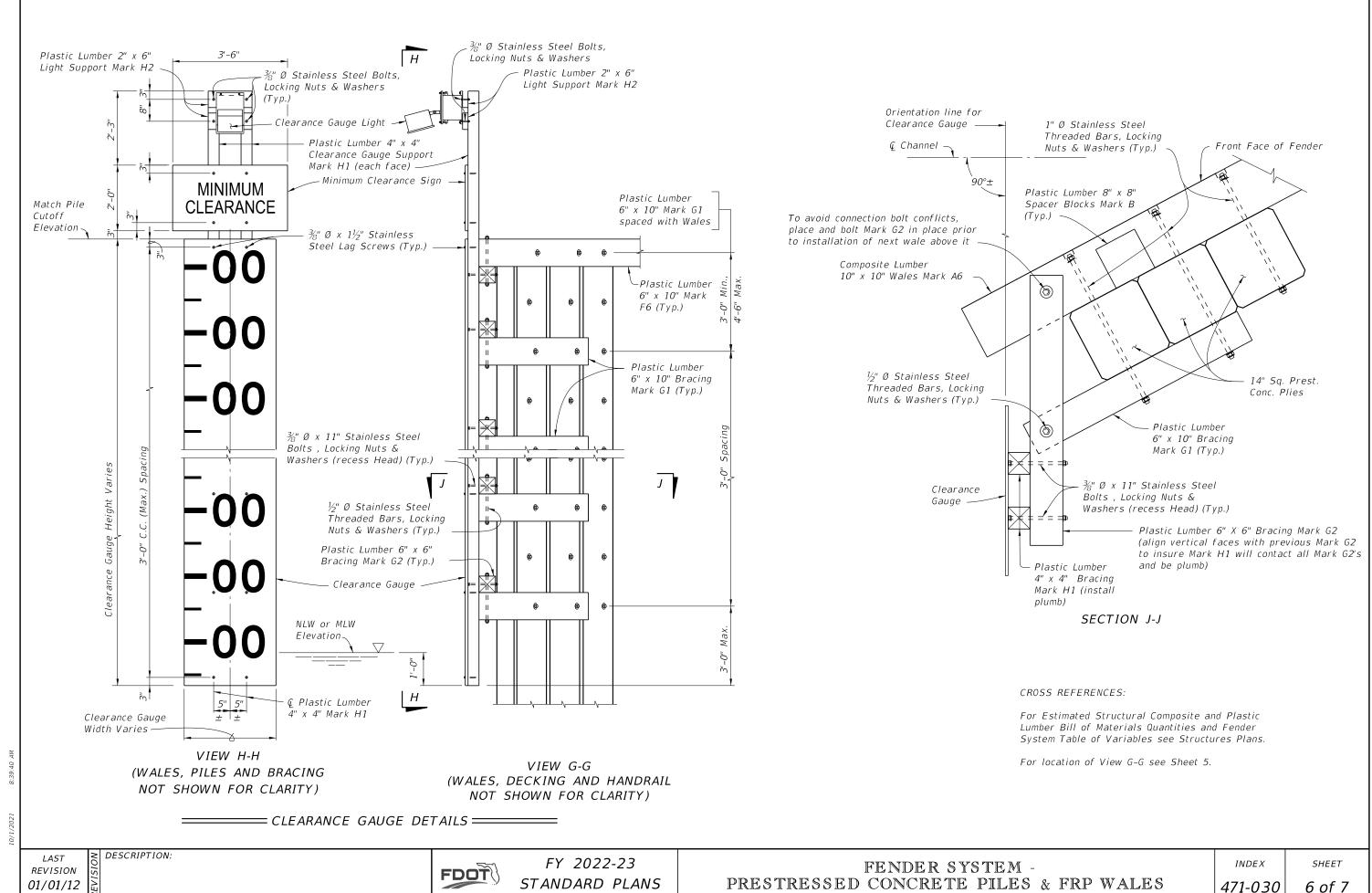
INDEX 471-030

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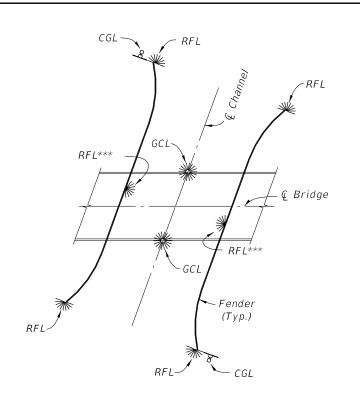


* STRUCTURAL COMPOSITE LUMBER BILL OF MATERIALS						
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY	
A1	10" X 10" COMPOSITE LUMBER	32'-0" (STRAIGHT)	266.6	nber		
A2	10" X 10" COMPOSITE LUMBER	32'-0"	266.6	mposite and Plastic Lumber e in Structures Plans	res Plans	
АЗ	10" X 10" COMPOSITE LUMBER	16'-0"	133.3		in Stru	
A4	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	d Structural Com Materials Table		
A5	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	See Estimated Structural Composite Bill of Materials Table in Stru		
A6	10" X 10" COMPOSITE LUMBER	16'-0"	133.3			

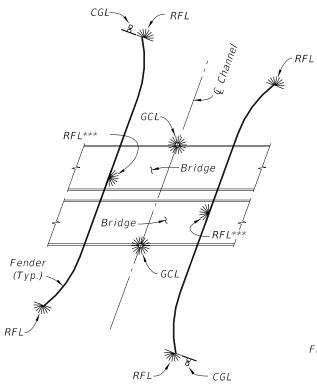
	*	PLASTIC LUMBER BILL OF MA	TERIALS			
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY	
В	8" X 8" PLASTIC LUMBER	8" (STRAIGHT)	3.6			
С	2" X 6" PLASTIC LUMBER	16'-0" (STRAIGHT) (Trim & Miter Ends as required)	16.0			
D	4" X 6" PLASTIC LUMBER	4'-4" (STRAIGHT)	8.7			
** E	2" X 12" PLASTIC LUMBER	2'-6" (STRAIGHT) (Miter as required, 6" Min. width)	5.0			
F 1	6" X 10" PLASTIC LUMBER	32'-0" (STRAIGHT)	160.0	I Plastic Lumber res Plans		
F2	6" X 10" PLASTIC LUMBER	31'-11"	159.6			
F3	6" X 10" PLASTIC LUMBER	15'-11"	79.6	Estimated Structural Composite and Plastic Lumber Bill of Materials Table in Structures Plans		
F4	6" X 10" PLASTIC LUMBER	15'-91/4"	78.8			
F5	6" X 10" PLASTIC LUMBER	15'-81/4"	78.4		Bill of Ma	
F6	6" X 10" PLASTIC LUMBER	15'-101/4"	79.3	See		
G 1	6" X 10" PLASTIC LUMBER	3'-8" (STRAIGHT)	18.3			
G2	6" X 6" PLASTIC LUMBER	4'-1" (STRAIGHT)	12.3			
H1	4" X 4" PLASTIC LUMBER	PILE CUTOFF ELEV. MINUS NLW OR MLW ELEV. PLUS 5'-6" (STRAIGHT)	1.3 PER LF EACH			
Н2	2" X 6" PLASTIC LUMBER	1'-2" (STRAIGHT)	1.2			

<sup>\*</sup> All Plastic Lumber and Composite Lumber Dimensions and Quantities shown are based on Nominal Lumber Dimensions and may vary depending on Actual Lumber Dimension.

<sup>\*\*</sup> Provide Fiberglass Open Grating in lieu of 2" X 12" Plastic Lumber when called for in the Plans. Mounting hardware shall be Stainless Steel, install per Manufacturer's recommendations. See Structures Plans for Notes and Details.



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITH FENDERS



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR DUAL BRIDGES WITH FENDERS

# NAVIGATION LIGHT NOTES: 1. Provide Navigation Light System in compliance with Specifications Section 510.

-RFL or RCL \*

FROM POWER SOURCE

NOTE: Size conduit and conductors per NEC requirements. Do not use conduit smaller than  $\frac{3}{4}$ " Ø.

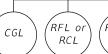
LC

Xmer

RCL

GCLGCL

CGL





### TYPICAL ELECTRICAL SCHEMATIC DIAGRAM

RFL or

RCL

RFL or

RCL

POWER CONDUCTORS				
DISTANCE	VOLTS	CONDUCTOR	TRANSFORMER	
(feet)				
0 - 75	120	#12 AWG	N/A	
75 - 500	120 or 240	#10 AWG	N/A	
500-1000	240	#10 AWG	N/A	
1000-2000	480	#10 AWG	2 KVA	
2000-5000	480	#8 AWG	2 KVA	
5000-10000	480	#6 AWG	2 KVA	
over 10000	480	#4 AWG	2 KVA	

### LEGEND

### SYMBOL DESCRIPTION

Lighting Contactor

Photocell Control

Xmer Transformer (If Required)

 $\lceil RFL \rceil$ or

Red Pier/Fender Light (180° visibility)

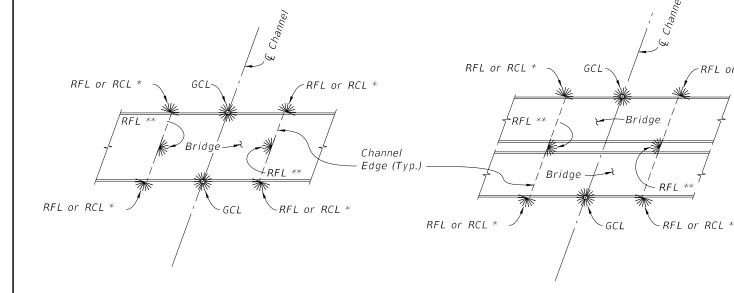
RCL

Red Channel Margin Light (180° visibility)

Green Center Channel Light (360° visibility)

△ CGL Clearance Gauge Light

> Channel Margin or Pier inner surface whichever defines Channel Edge.



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITHOUT FENDERS

NAVIGATION LIGHT SYSTEM SCHEMATIC FOR DUAL BRIDGES WITHOUT FENDERS

- \* Use RFL when Pier is at Channel Edge and see CFR, Title 33, part 118 for Mounting Height restrictions. Use RCL otherwise.
- \*\* Mounted only on the Pier that defines CM, otherwise does not apply.
- \*\*\* RFL to be located at mid length of straight portion of fender.

**REVISION** 11/01/17

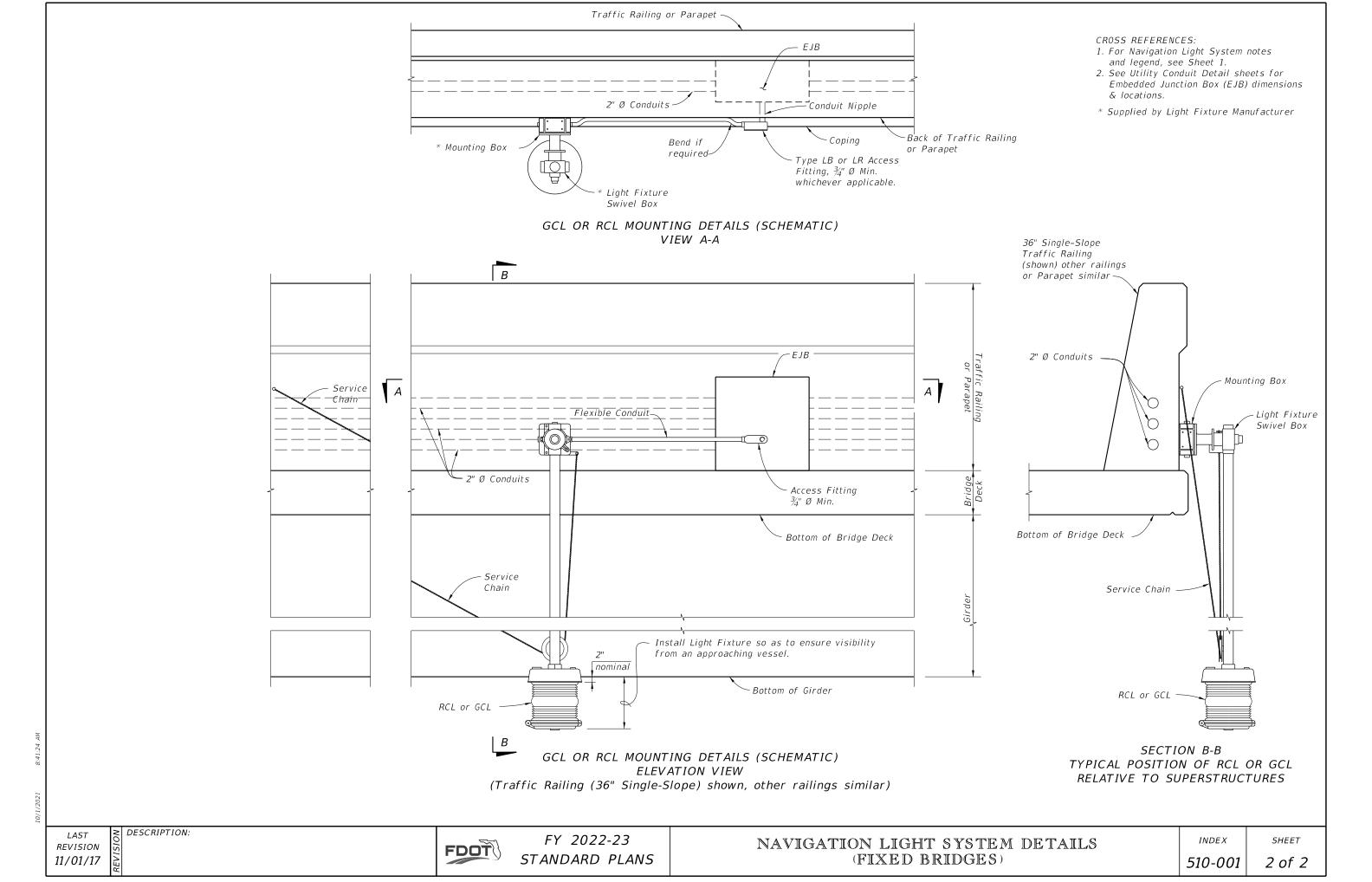
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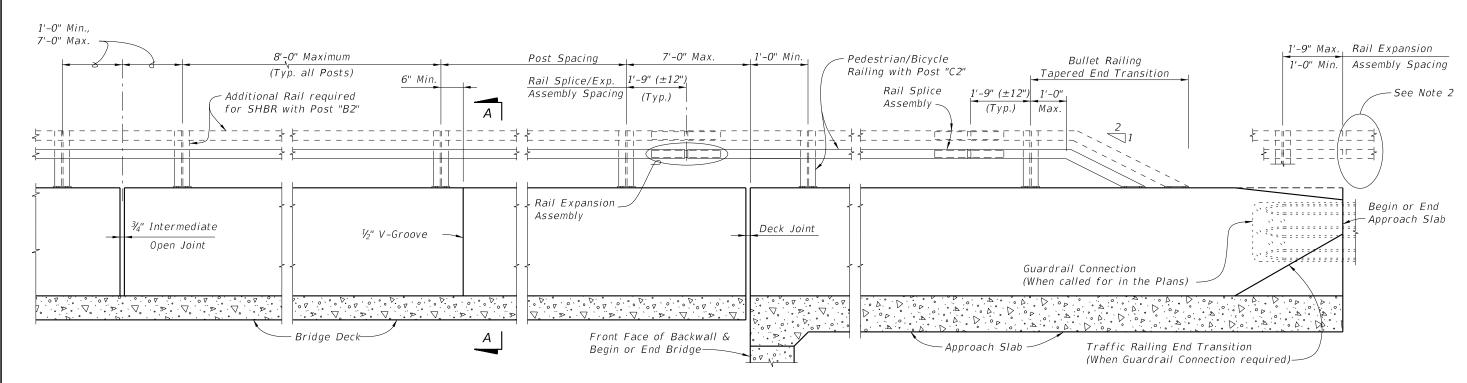
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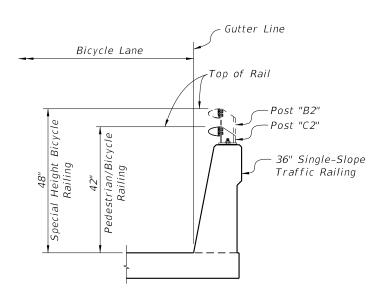
(FIXED BRIDGES)

INDEX 510-001 SHEET





### ELEVATION OF INSIDE FACE OF TRAFFIC RAILING WITH PEDESTRIAN/BICYCLE BULLET RAILING



SECTION A-A TYPICAL SECTION THRU BRIDGE DECK (APPROACH SLAB SIMILAR)

### NOTES:

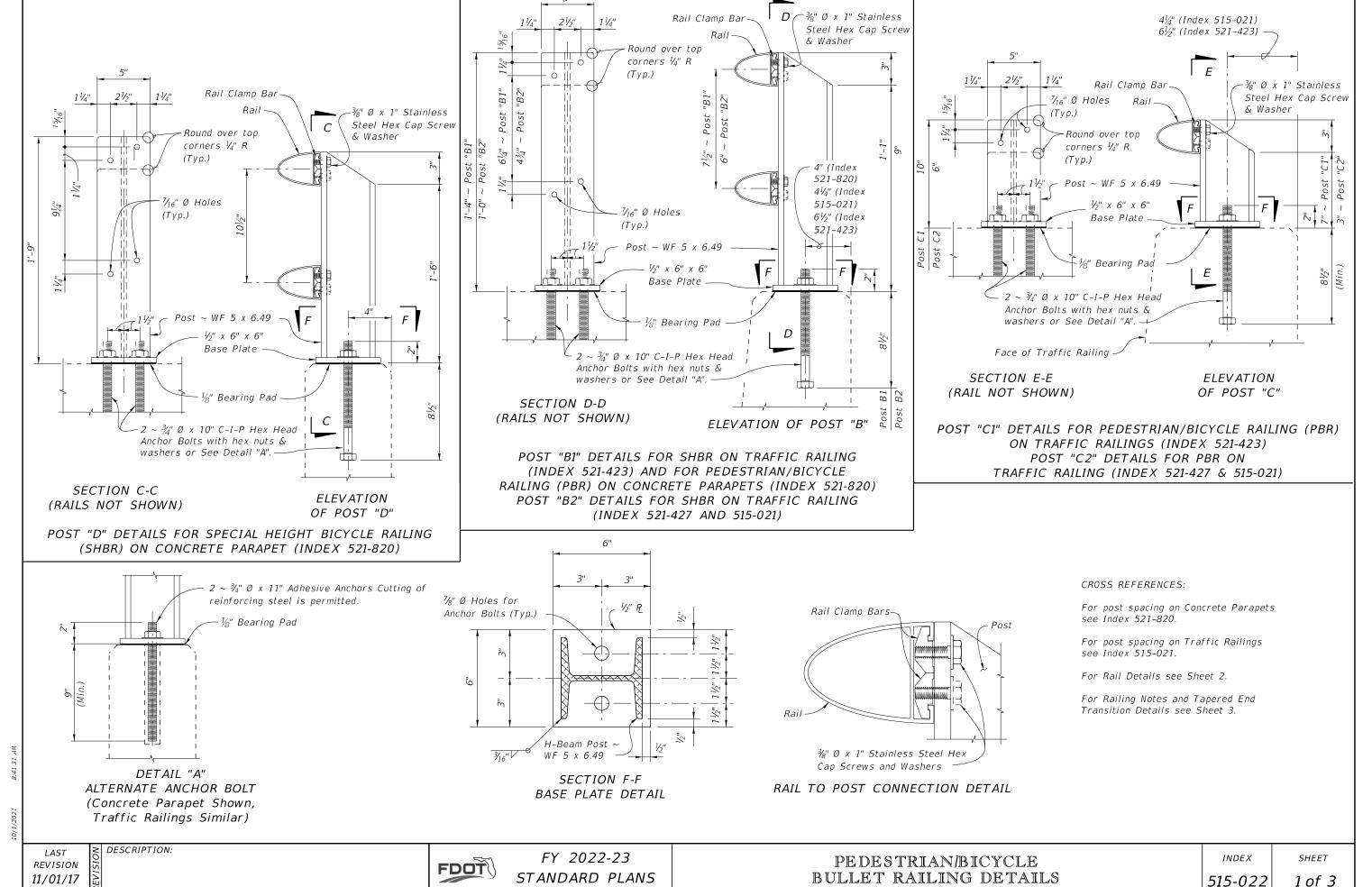
- 1. A Bullet Railing Tapered-End Transition is required for all approach ends of Bullet Railings on Traffic Railings. When Guardrail Connection is required terminate the Bullet Railing Tapered-End Transition at beginning of the Traffic Railing End Transition.
- 2. Where Bullet Railing continues on retaining wall mounted Traffic Railings or Barriers, provide a Bullet Railing Tapered End Transition at the terminus of the Bullet Railing.

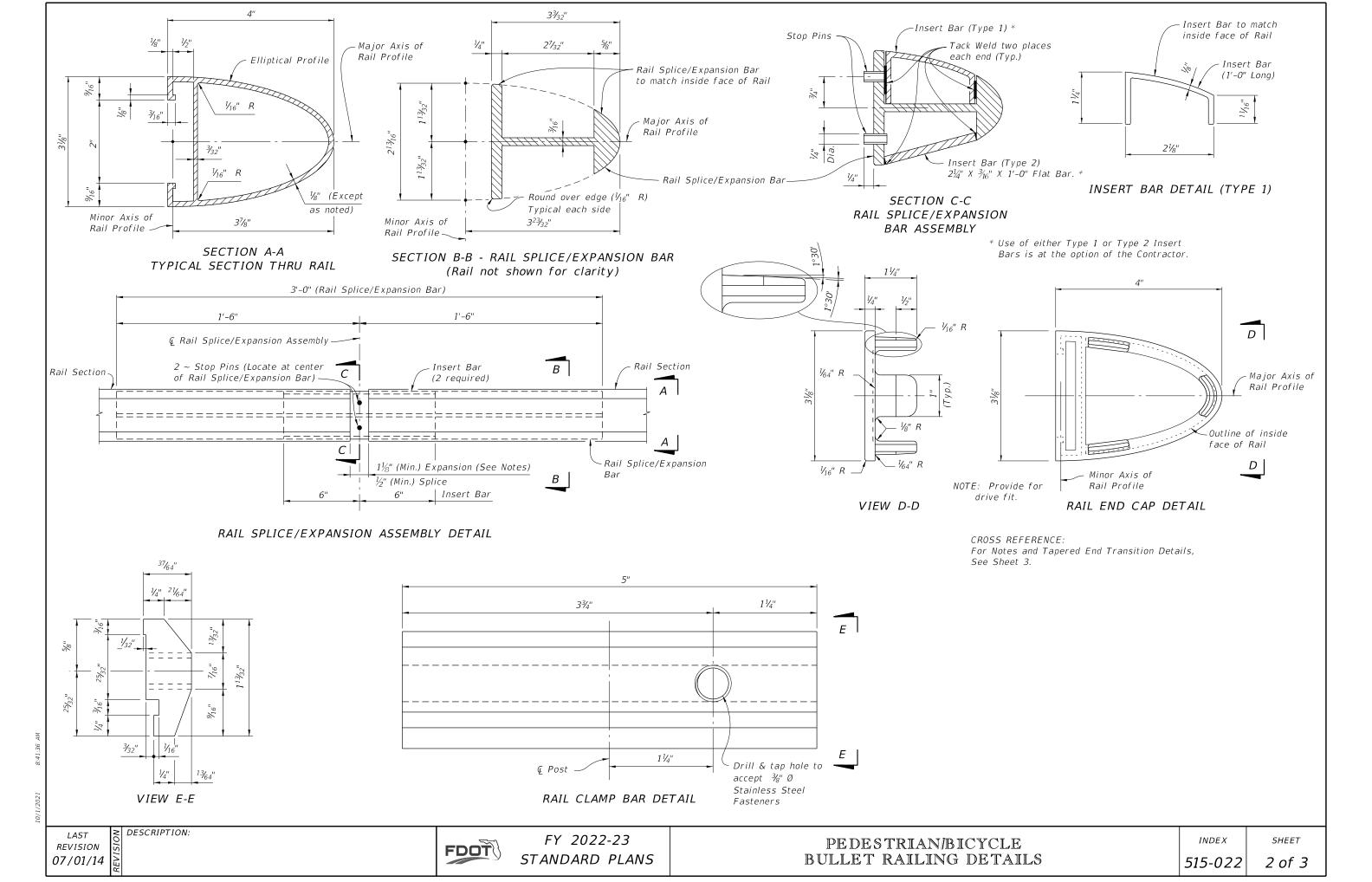
### CROSS REFERENCES:

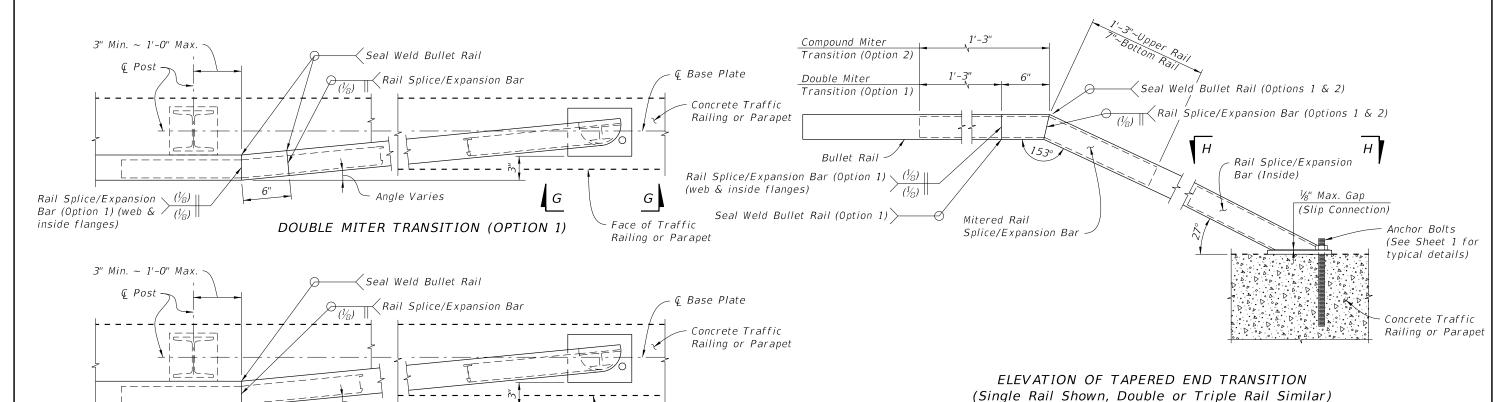
Work in conjunction with Index 515-022.

For Traffic Railing Details, Reinforcement and Notes see Index 521-427.

11/01/17







### PARTIAL PLAN OF TAPERED END TRANSITIONS

COMPOUND MITER TRANSITION (OPTION 2)

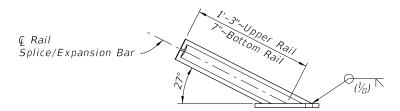
Angle Varies

G

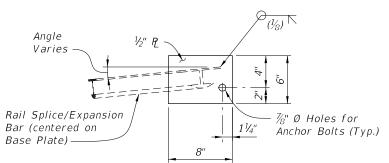
Face of Traffic

Railing or Parapet

(Single Rail Shown, Double or Triple Rail Similar)



### VIEW G-G TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)



VIEW H-H TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)

### RAILING NOTES:

1. Work this Index with Index 521-423, 521-427, 521-428, 521-820 and 515-021 and Specification Section 515.

Shop Drawings: Submit shop drawings prior to fabrication. A. Include post and rail splice/expansion assembly location for curved alignments with radii < 40 feet and for all end terminations.

A. Supply Aluminum materials In accordance with Specification Section 965 and the following. Wrought Aluminum Post: ASTM B221, Alloy 6061-T6 or 6351-T5 Rail End Cap: ASTM B26 sand cast aluminum alloy 356.0-F Plate and Bars: ASTM B209 Alloy 6061-T6 Rails: ASTM B221 Alloy 6061-T6 or 6351-T5. Stop Pins: Press-fit aluminum or stainless steel pins or tubes

B. Stainless Steel Fasteners: ASTM F-593, Alloy Group 2 (316).

C. Bearing Pads: Plain or Fiber Reinforced meeting Specification Section 932 for Ancillary Structures.

D. Anchor Bolts: Galvanized ASTM A307 Grade 36 Hex Head. Galvanized ASTM 1554 Grade 55 Threaded rods for Adhesive Anchors.

4. Layout.

A. Posts shall be uniformly spaced with reasonable consistency.

B. Tapered End Transitions are required at the terminus of the approach ends of Bullet Railing mounted on a Traffic Railing. Bullet Railings on concrete parapets shielded by a traffic railing do not require Tapered End Transitions unless noted otherwise in the Plans.

C. Adjust post spacing's to avoid parapet obstacles, such as armor expansion plates, by 9 inches minimum.

D. Rails shall be continuous over a minimum of 3 posts, except that lengths less than 12 feet need only be continuous over 2 posts.

E. Space splices at 40 feet maximum. Splice all rails in a given railing section at about the same center line.

F. Provide rail expansion assemblies in panels between posts on either side of a bridge expansion joint. Rail expansion assemblies are similar to the rail splice assemblies with increased space at the expansion assembly to allow for movement equal to 1.5 times the bridge joint opening or 1" greater than the expected joint movement.

5. Installation:

A. Set rails near bridge expansion joints to allow for expected movement.

B. Cutting of reinforcing steel is permitted for post installed anchors.

6. Payment: Includes the full cost of installed bullet railing. Cost of the Concrete Parapet or Traffic Railing is separate.

**REVISION** 11/01/20

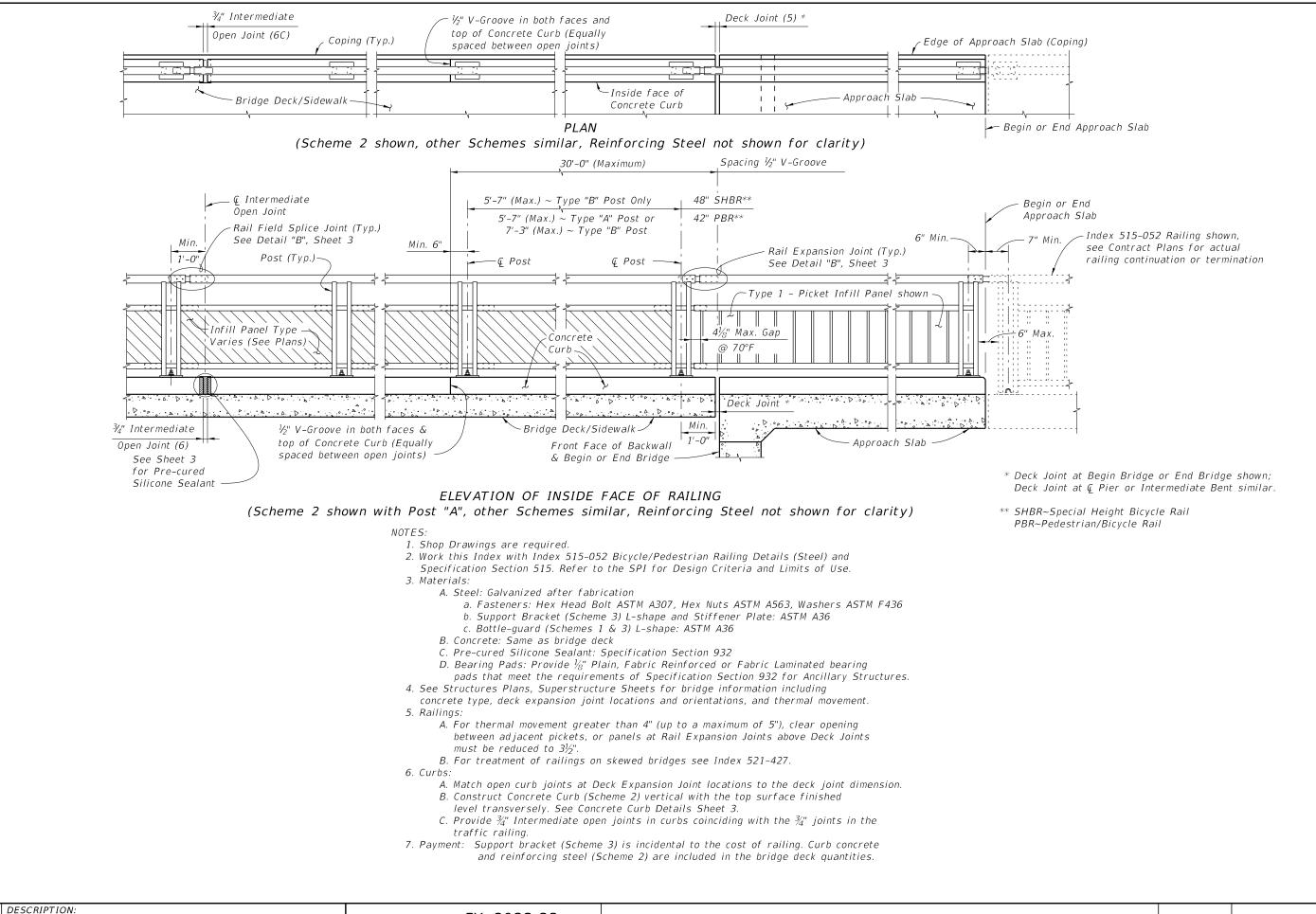
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FY 2022-23 STANDARD PLANS

PEDESTRIAN/BICYCLE BULLET RAILING DETAILS INDEX

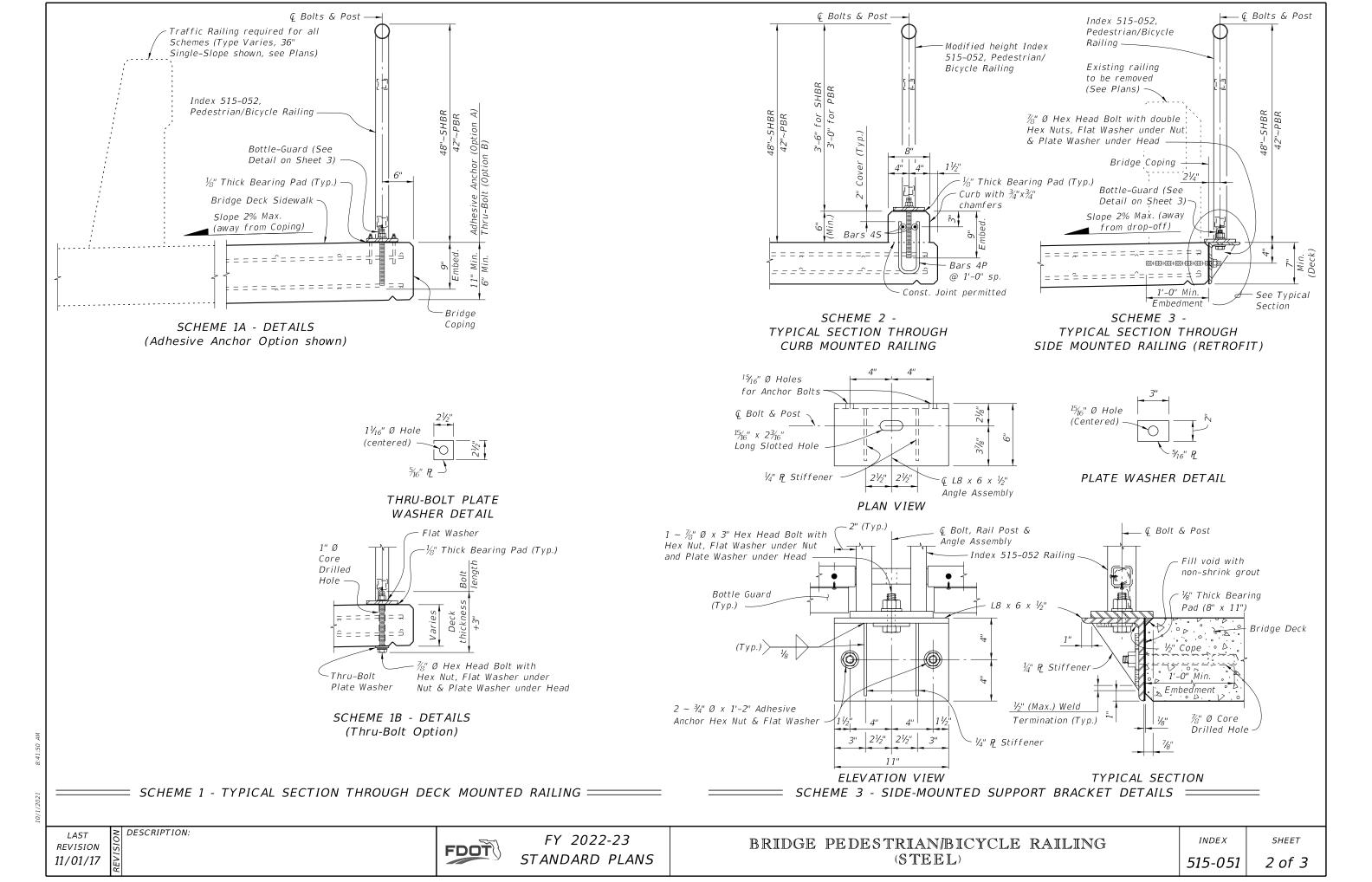
SHEET

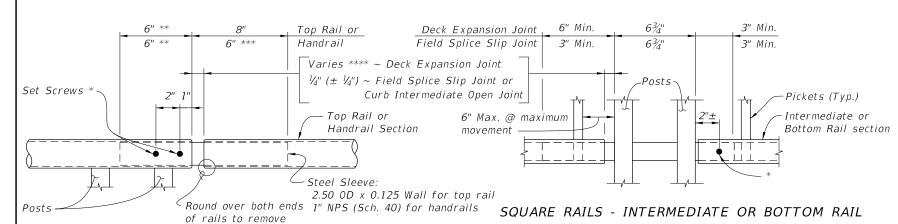
515-022 3 of 3



10/1/2021

LAST REVISION 11/01/17



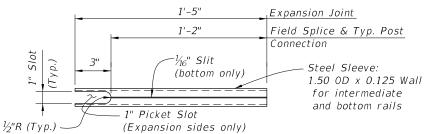


## ROUND RAILS - TOP RAIL OR HANDRAIL

- \*  $\frac{1}{4}$ " Ø x  $\frac{3}{4}$ " Pan Head Stainless Steel (Type 316 or 18–8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A  $\frac{3}{4}$ " Ø plug weld may be substituted for the two set screws at expansion joints.
- \*\* Embedded length may be 4" for plug welded connection.
- \*\*\* Increase handrail sleeve embedment to 8" for Expansion Joint openings greater than 2".

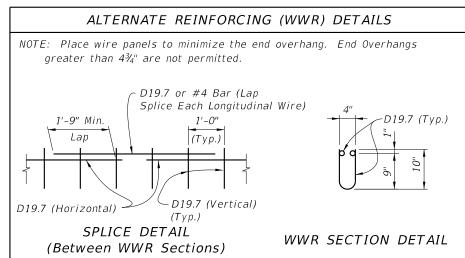
sharp edges (Typ.)

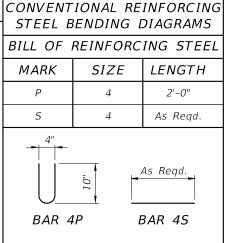
\*\*\*\* Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".



INTERMEDIATE OR BOTTOM RAIL - STEEL SLEEVE DETAIL (Bottom Side Shown)

## = DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =





#### CURB REINFORCING STEEL NOTES:

ESTIMATED CONCRETE CURB

QUANTITIES (SCHEME 2)

UNIT

CY/LF

LB/LF

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".

QUANTITY

0.0124

4.01

5. Deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931 may be used in lieu of all Bars 4P and 4S.

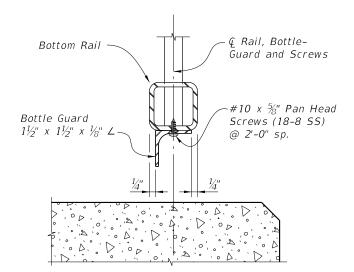
Pre-cured Silicone Sealant (4" wide)	6" Min. 34" Cha	mfer (Typ.)
over.		
-	2"	
ents and 4S.		

## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTE:

At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

## SCHEME 2 - CONCRETE CURB DETAILS =

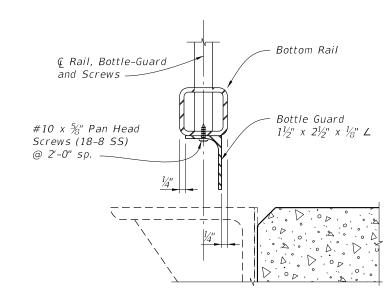


TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 1 - BOTTLE GUARD DETAIL =

#### CROSS REFERENCE:

See Sheet 1 for Bridge Railing Notes.



TYPICAL SECTION THROUGH BOTTOM RAIL
(Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL ===

LAST REVISION 11/01/16

FDOT

FY 2022-23 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING (STEEL)

INDEX

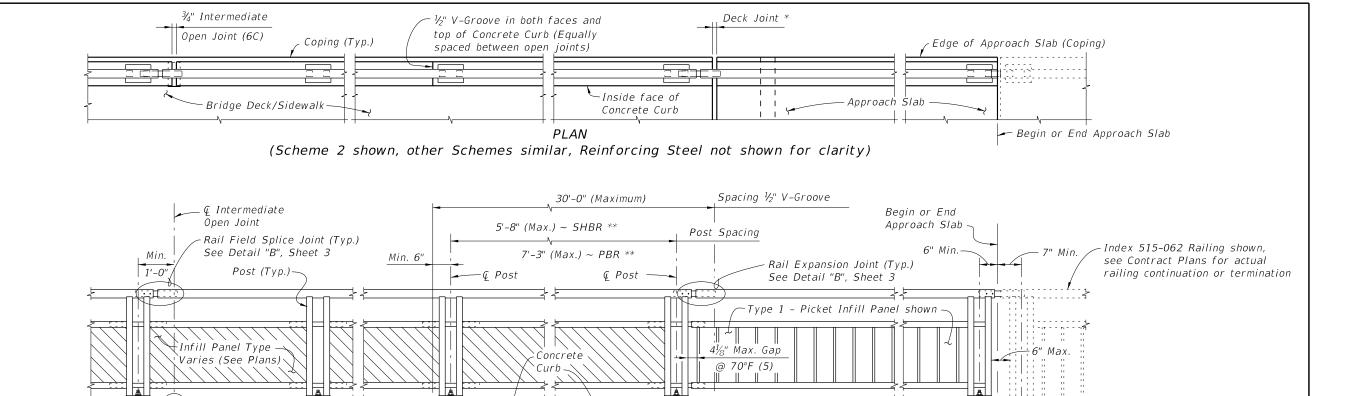
SHEET

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ITEM

Reinforcing Steel

Concrete



Min.

NOTES:

½" V-Groove in both faces &

top of Concrete Curb (Equally

spaced between open joints)

¾" Intermediate

Open Joint (6)

See Detail "A", Sheet 3 for Pre-cured

Silicone Sealant

- 2. Work this Index with Index 515-062 Aluminum Bicycle/Pedestrian Railing Details and Specification Section 515. Refer to the IDS for Design Criteria and Limits of Use.
- 3. Materials:
  - A. Galvanized Steel Fasteners: Hex Head Bolt ASTM A307, Hex Nuts ASTM A563, Washers ASTM F436
  - B. Aluminum:

1. Shop Drawings are required.

- a. Support Bracket (Scheme 3) L-shape and Stiffener Plate: ASTM B209, Alloy 6061-T6
- b. Bottle-guard (Schemes 1 & 3) L-shape: ASTM B209, Alloy 6061-T6 or 6063-T5
- C. Concrete: Same as bridge deck
- D. Pre-cured Silicone Sealant: Specification Section 932
- E. Bearing Pads: Provide  $\frac{1}{8}$ " thick Plain, Fabric Reinforced or Fabric Laminated pads meeting the requirements of Specification Section 932 for Ancillary Structures.
- 4. See Structures Plans, Superstructure Sheets for bridge information including concrete type, deck expansion joint locations and orientations, and thermal movement.

Bridge Deck/Sidewalk~

ELEVATION OF INSIDE FACE OF RAILING

(Scheme 2 shown with Post "A", other Schemes similar, Reinforcing Steel not shown for clarity)

Front Face of Backwall

& Begin or End Bridge

- 5. Railings:
  - $\tilde{A}$ . For thermal movement greater than 4" (up to a maximum of 5"), clear opening between adjacent pickets, or panels at Rail Expansion Joints above Deck Joints must be reduced to  $3\frac{1}{2}$ ".
  - B. For treatment of railings on skewed bridges see Index 521-427.
- 6. Curbs:
  - A. Match open curb joints at Deck Expansion Joint locations to the deck joint dimension.
  - B. Construct Concrete Curb (Scheme 2) vertical with the top surface finished level transversely. See Concrete Curb Details Sheet 3.
  - C. Provide 3/4" Intermediate open joints in curbs coinciding with the 3/4" joints in the traffic railing.
- 7. Payment: Support bracket (Scheme 3) is incidental to the cost of railing. Curb concrete and reinforcing steel (Scheme 2) are included in the bridge deck quantities.

10/1/2021

LAST REVISION 11/01/17

DESCRIPTION:

ρ . . Δ . . Δ . D . o . ·

Approach Slab

Deck Joint \*

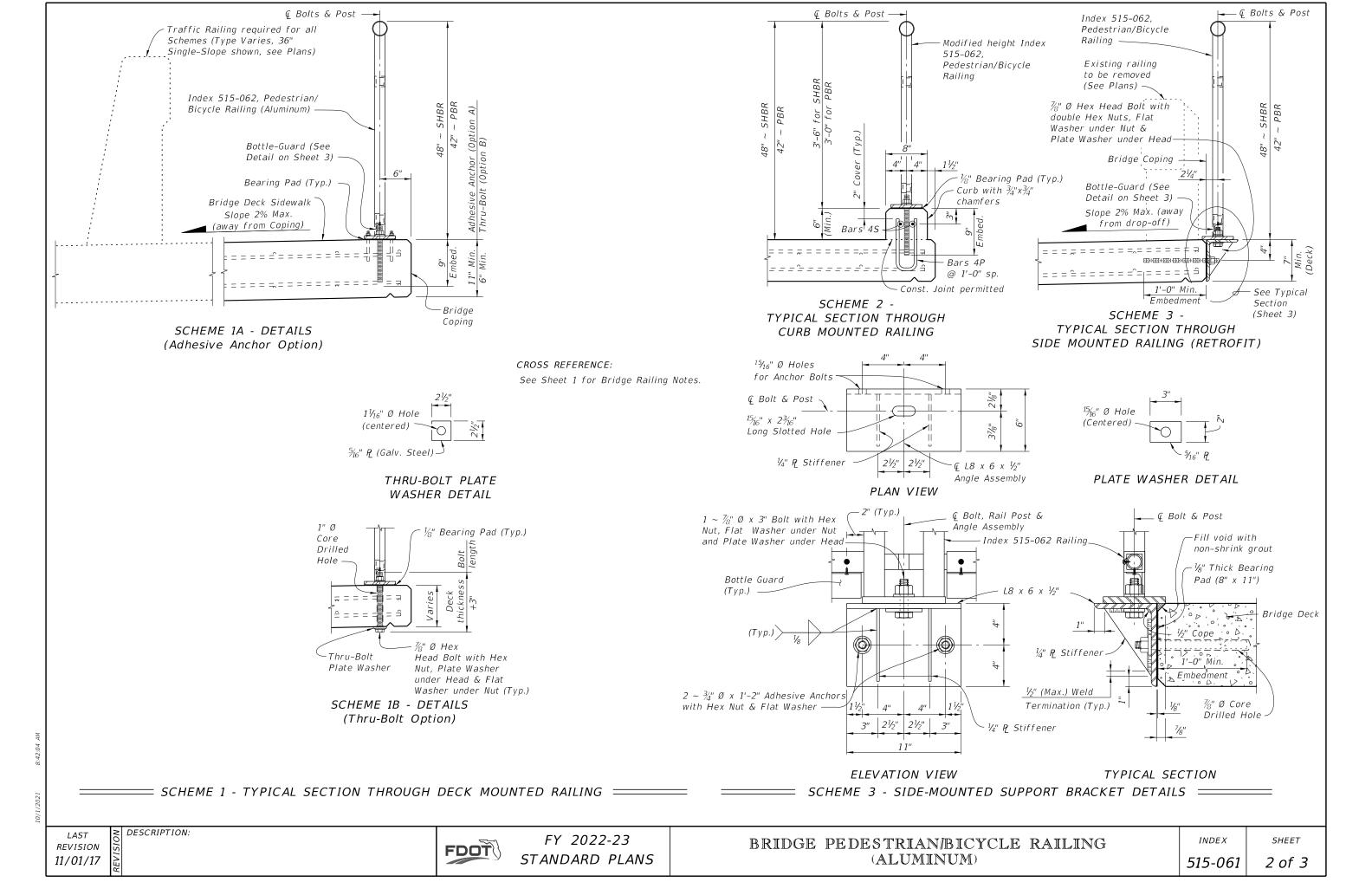
\* Deck Joint at Begin Bridge or End Bridge shown;

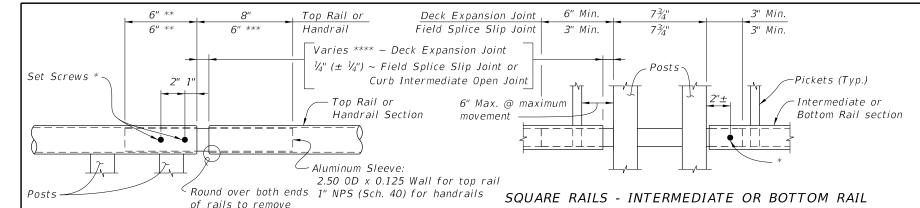
\*\* SHBR ~ Special Height Bicycle Railing

PBR ~ Pedestrian/Bicycle Railing

Deck Joint at & Pier or Intermediate Bent similar.

061 1 of 3



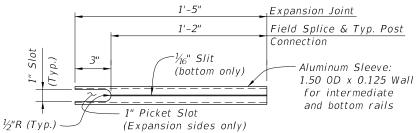


## ROUND RAILS - TOP RAIL OR HANDRAIL

\*  $\frac{1}{4}$ " Ø x  $\frac{3}{4}$ " Pan Head Aluminum (Alloy 7075-T73) or Stainless Steel (Type 316 or 18-8 Alloy) Set Screws along outside face of railing Set screws must be set flush against the rail surface. A  $\frac{3}{4}$ " Ø plug weld may be substituted for the two set screws at expansion joints.

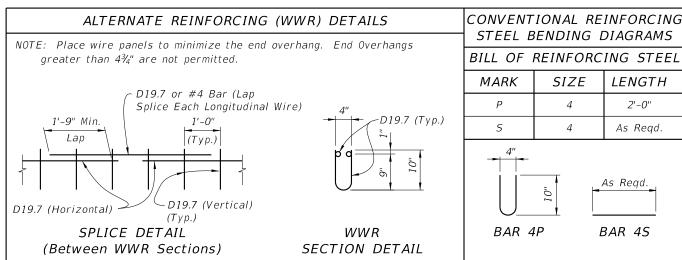
sharp edges (Typ.)

- \*\* Embedded length may be 4" for plug welded connection.
- \*\*\* Increase handrail sleeve embedment to 8" for Expansion Joint openings greater than 2".
- \*\*\*\* Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".



INTERMEDIATE OR BOTTOM RAIL - ALUMINUM SLEEVE DETAIL (Bottom Side Shown)

## DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =



#### CURB REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Deformed WWR meeting the requirements of Specifications Section 931 may be used in lieu of all Bars 4P and 4S.

Pre-cured Silicone Sealant (4" wide)	6" Min. 34" Chamfer (Typ.
over.	-
n 931	2"

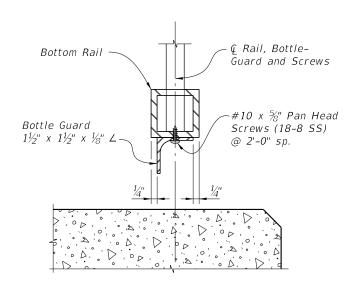
## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

#### ESTIMATED CONCRETE CURB QUANTITIES (SCHEME 2) ITFM UNIT **QUANTITY** CY/LF0.0124 Concrete Reinforcing Steel LB/LF 4.01

#### INTERMEDIATE JOINT SEAL NOTE:

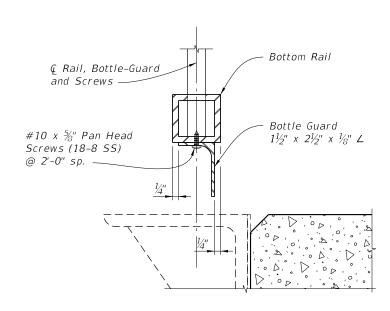
At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

## SCHEME 2 - CONCRETE CURB DETAILS =



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 1 - BOTTLE GUARD DETAIL =



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL =

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

INDEX 515-061

SHEET

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## GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

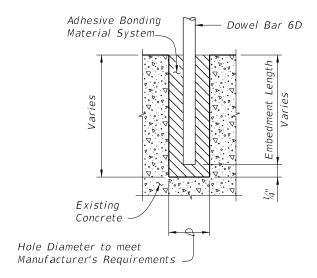
BRIDGES ON CURVED ALIGNMENTS: The details presented in this Standard are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing along the entire length of the bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

GUARDRAIL: See Index 536-001 for guardrail component details, geometric layouts and associated notes not fully detailed

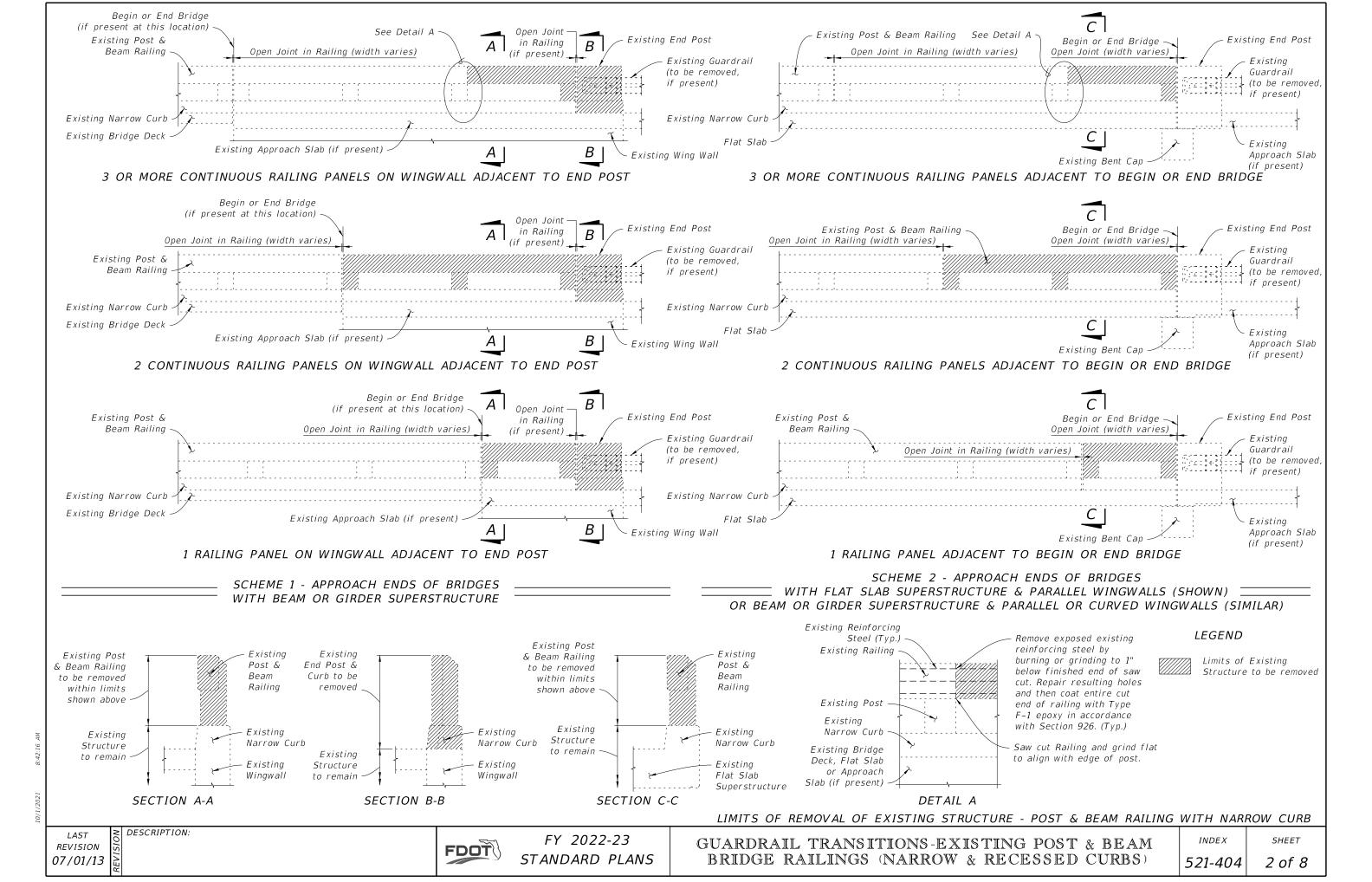
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise individual decals of letters

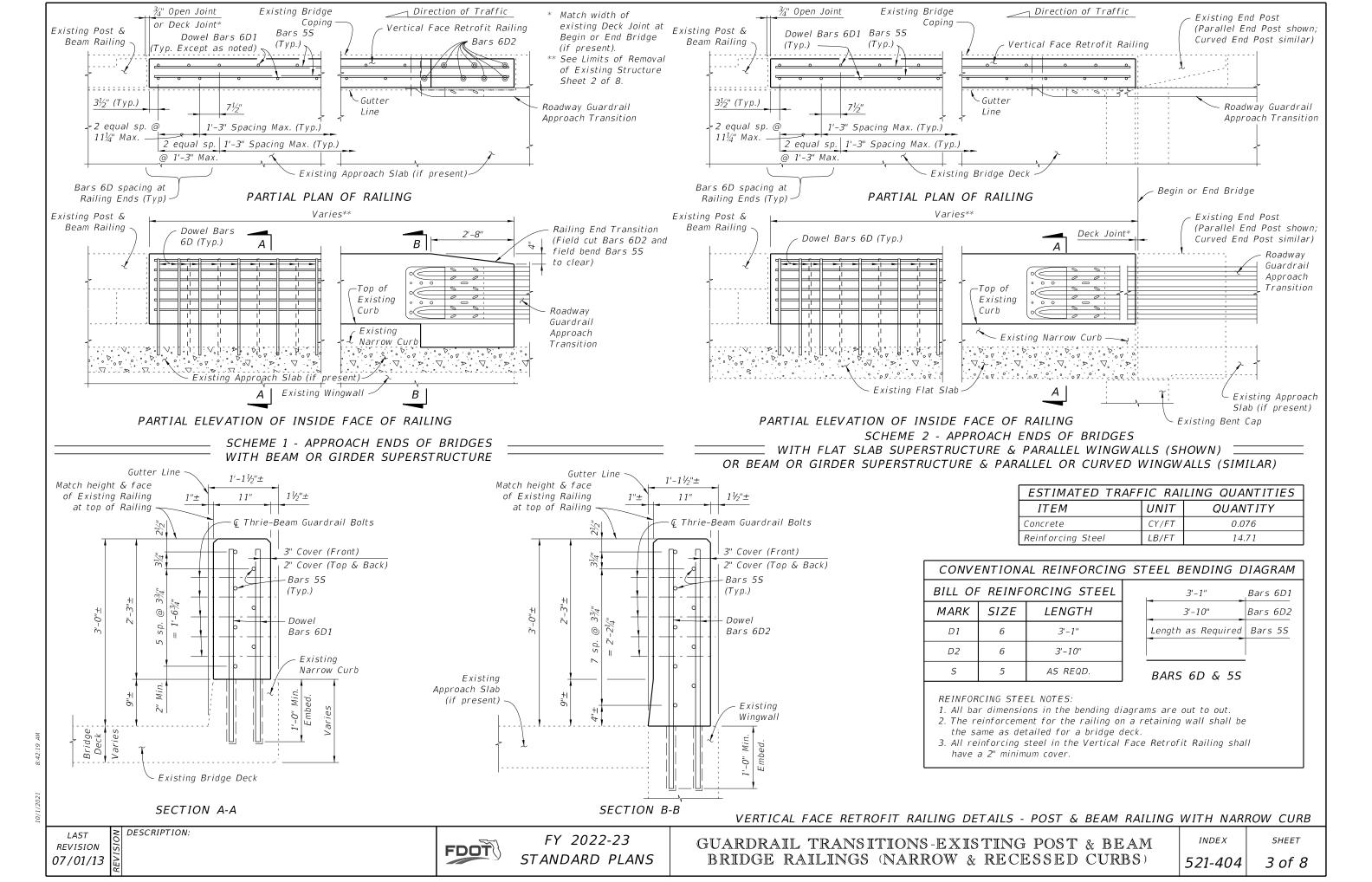
PAYMENT: Concrete Traffic Railing-Bridge Retrofit - Post & Beam Railing (EA) includes all material and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railing. Guardrail Approach Transition to rigid Barriers (EA) includes transition block, and necessary hardware to complete the Guardrail transitions shown.

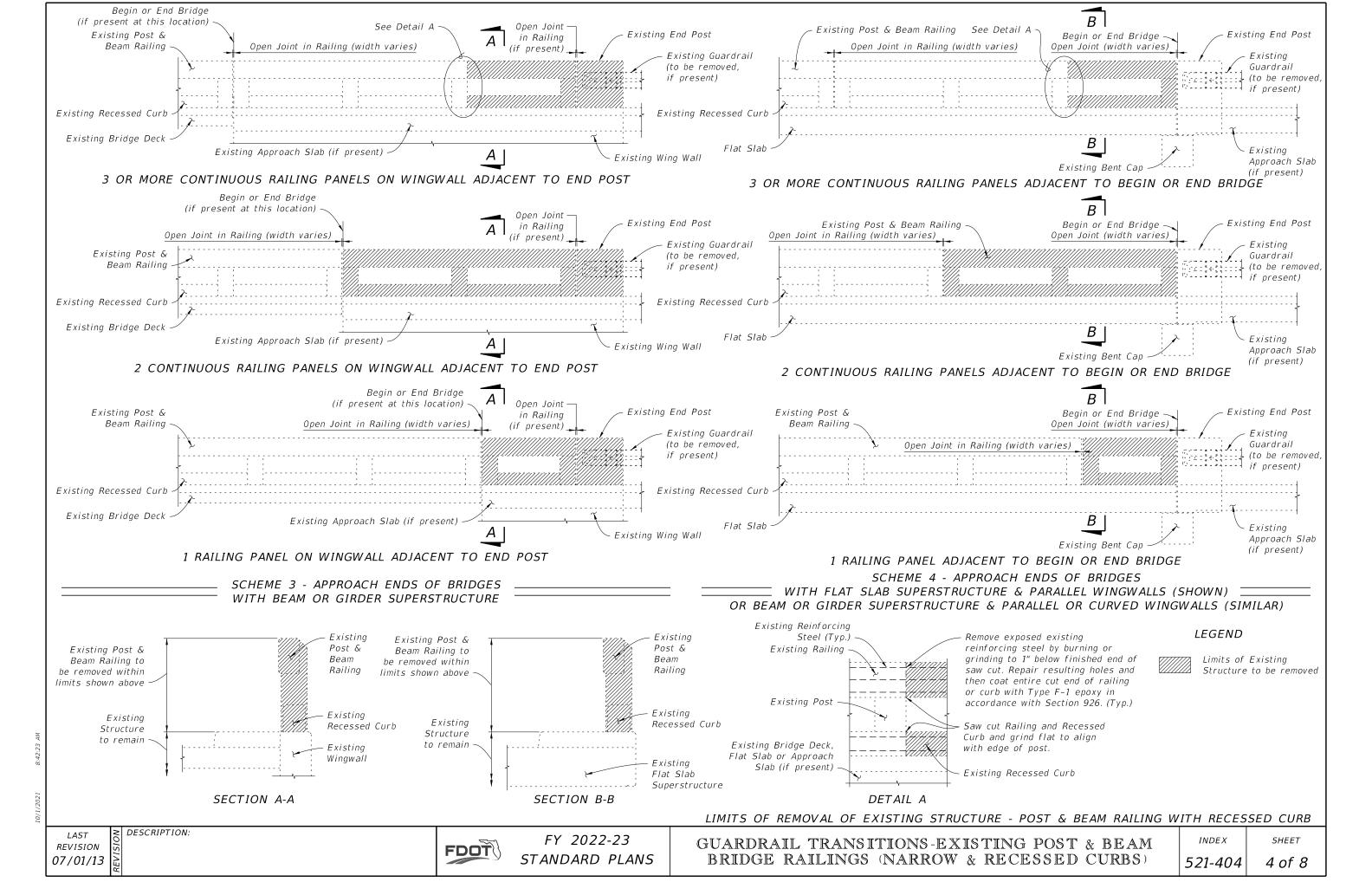


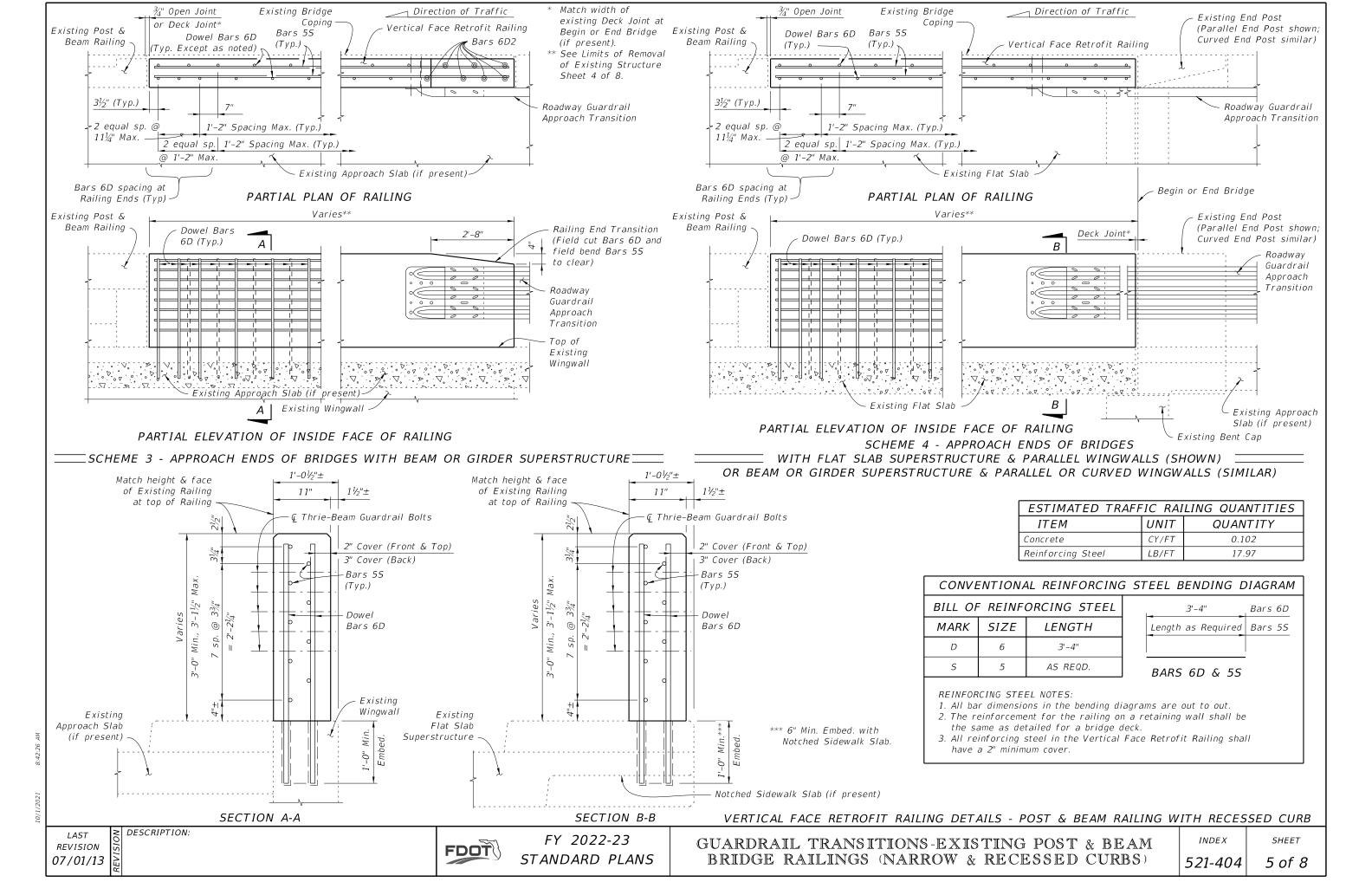
#### DOWEL DETAIL

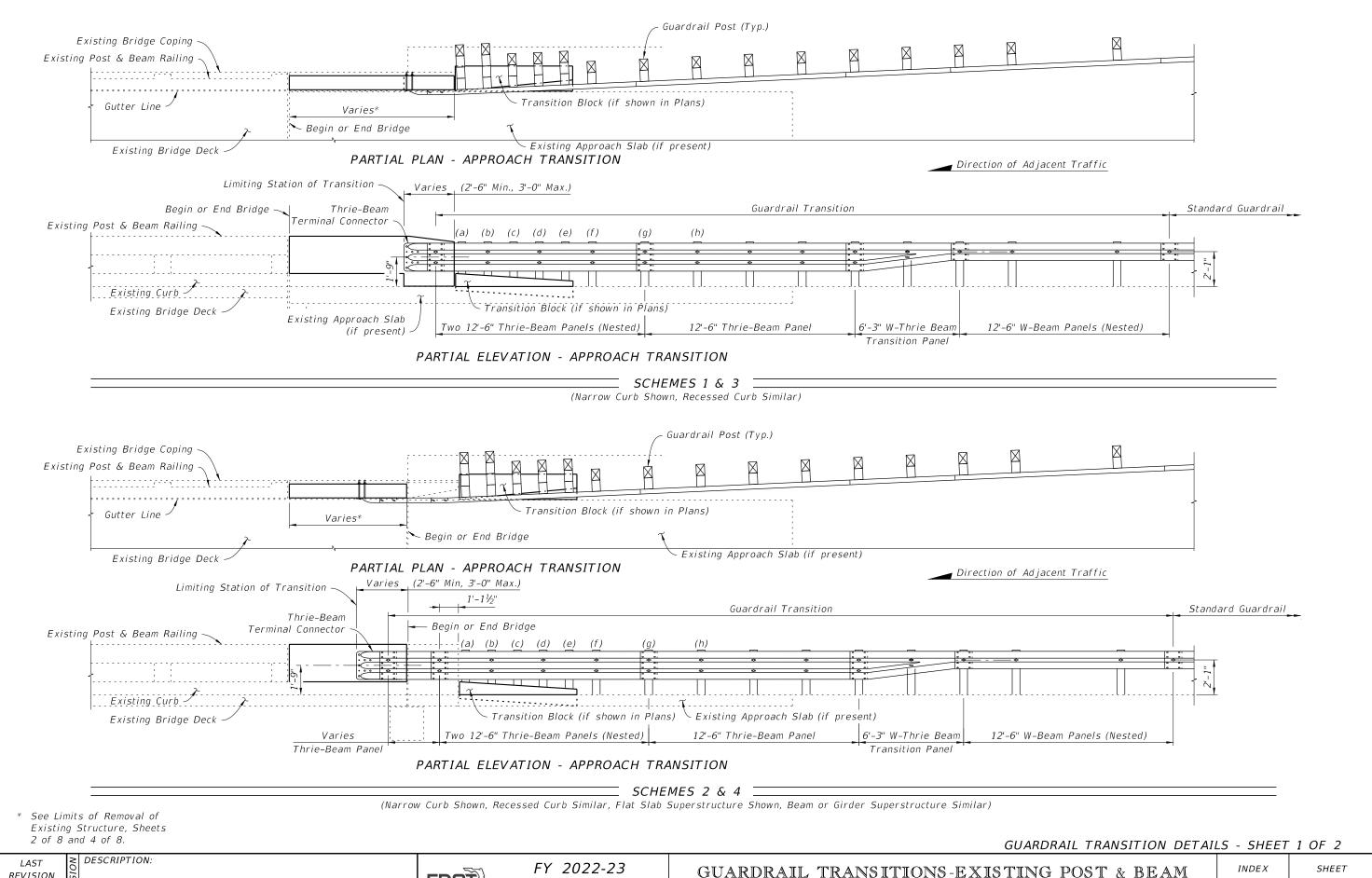
Note: Shift dowel holes to clear if the existing reinforcement is encountered.











**REVISION** 07/01/14

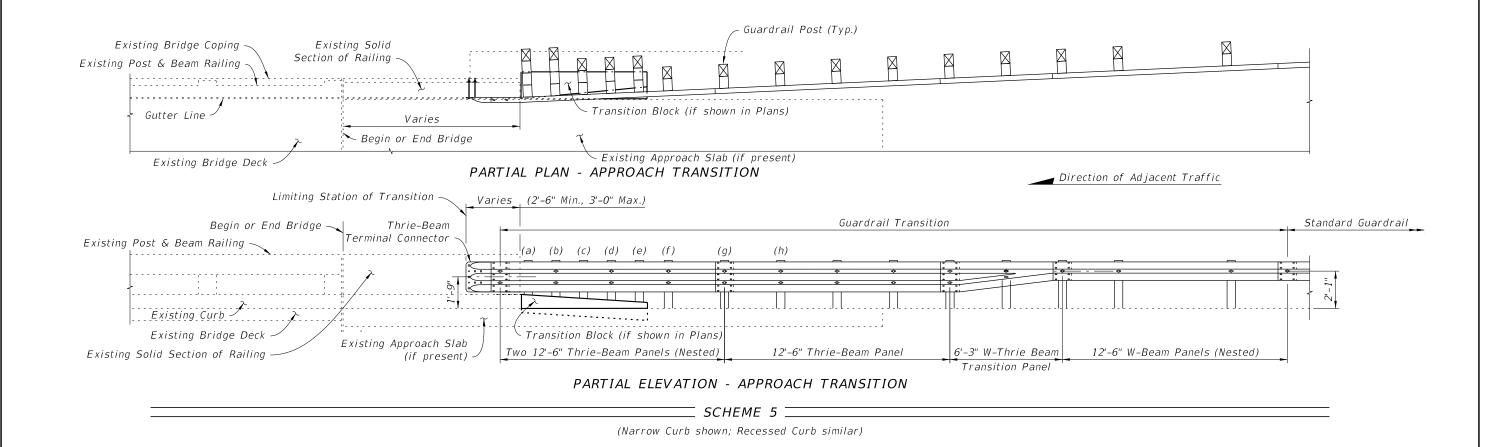
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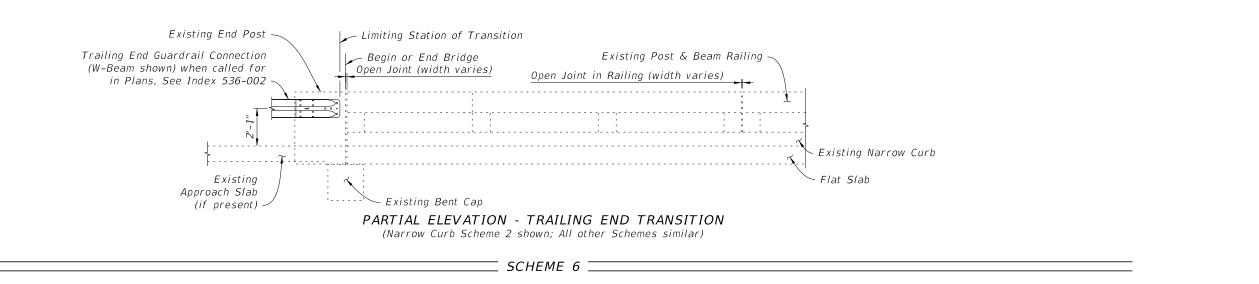
STANDARD PLANS

BRIDGE RAILINGS (NARROW & RECESSED CURBS)

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DESCRIPTION: LAST **REVISION** 

07/01/14

FDOT

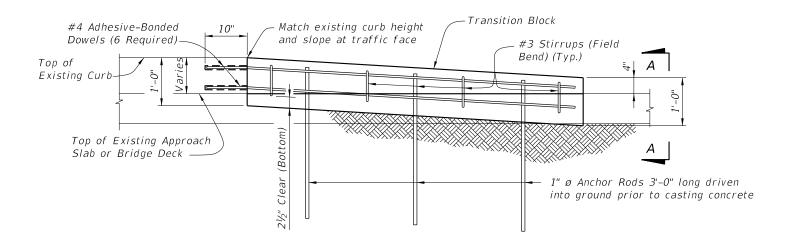
FY 2022-23 STANDARD PLANS GUARDRAIL TRANSITIONS-EXISTING POST & BEAM

INDEX *521-404* 

GUARDRAIL TRANSITION DETAILS - SHEET 2 OF 2

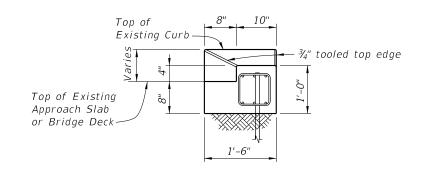
SHEET 7 of 8

PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

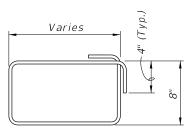


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK		
ITEM UNIT QUANTITY		
Concrete Class II (Bridge Deck)	CY	0.4
Reinforcing Steel	LB	61



END VIEW A-A



**#3 STIRRUP (FIELD BEND)** 

### NOTES:

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

ADHESIVE-BONDED DOWELS: Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

#### GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

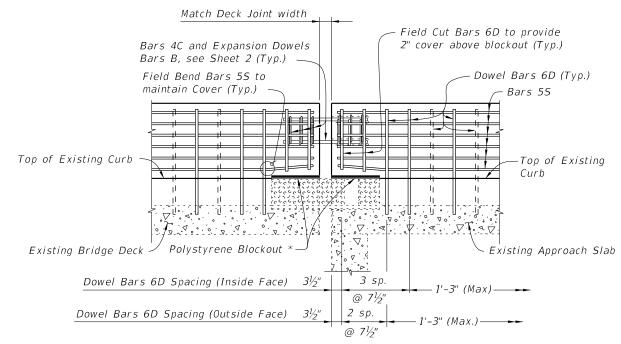
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install barrier delineators on top of the Traffic Railing along the entire length of bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Concrete Traffic Railing - Bridge Retrofit - Post & Beam Railing (each) includes all materials and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railings. Guardrail Approach Transition to Rigid Barriers (EA) includes all transition blocks, and necessary hardware to complete the Guardrail transitions shown.

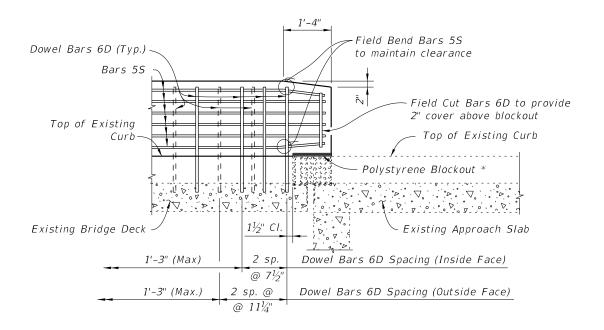
ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM UNIT		QUANTITY	
I I EM	UNIT	9" Curb	Increment
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.)



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEMES 2 THRU 5

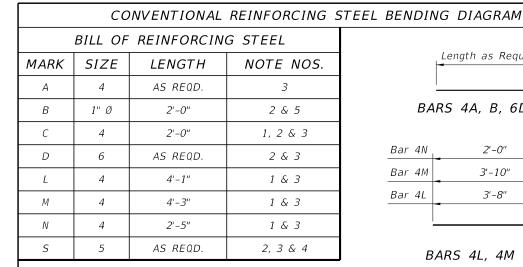
\* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEME 1 (Guardrail Transition not shown for clarity)

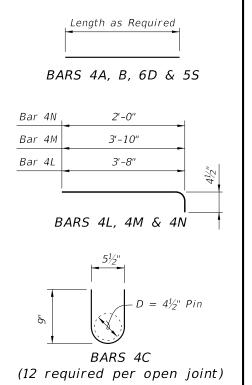
0.67.0

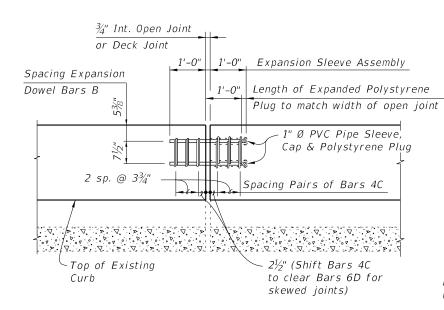
LAST REVISION 11/01/19



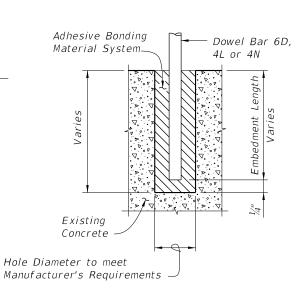
#### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.





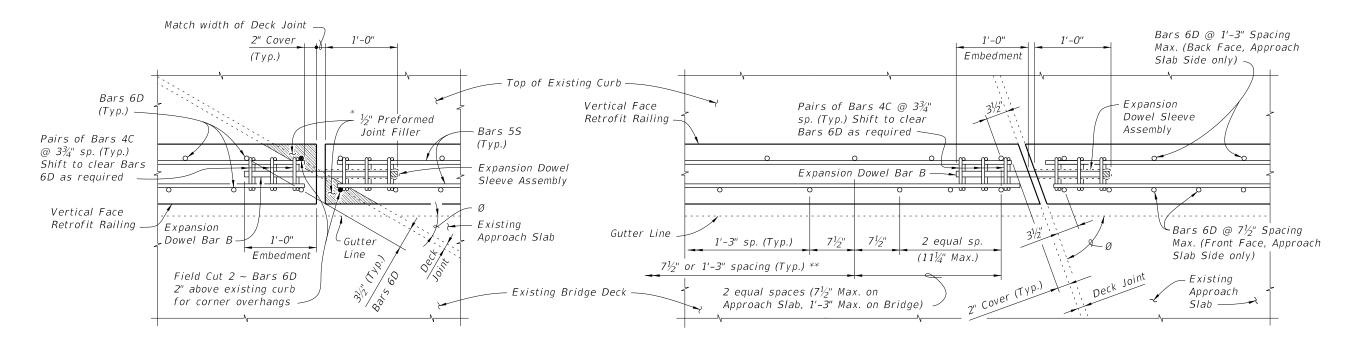
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



## DOWEL DETAIL

Dowel Installation Note: Shift dowel holes to clear if the existing reinforcement is encountered.

\*  $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.



SKEW DETAIL =

REVISION 07/01/13

DESCRIPTION:

**FDOT** 

PARTIAL PLAN OF RAILING (SKEW ANGLE Ø LESS THAN 70°)

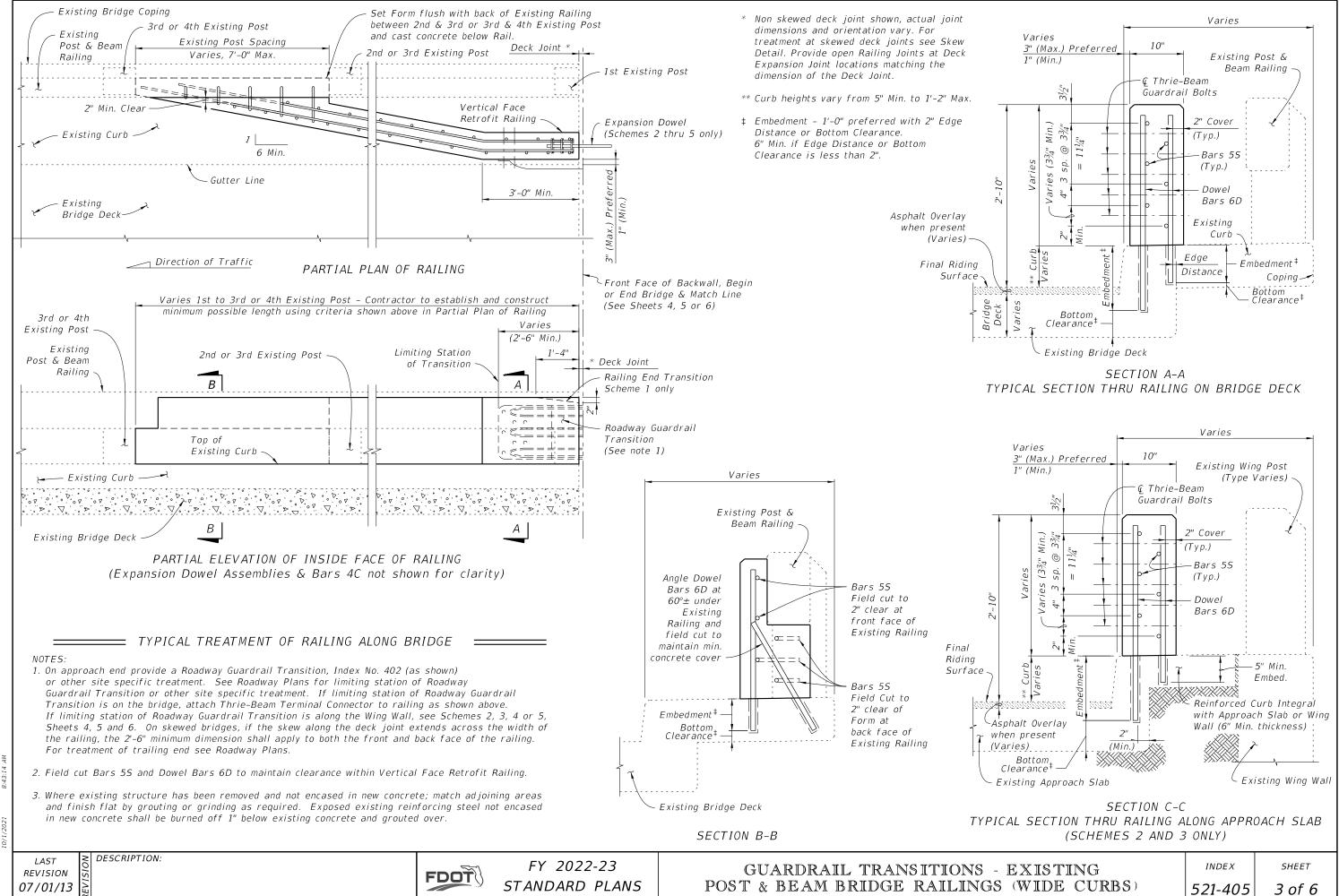
FY 2022-23 STANDARD PLANS

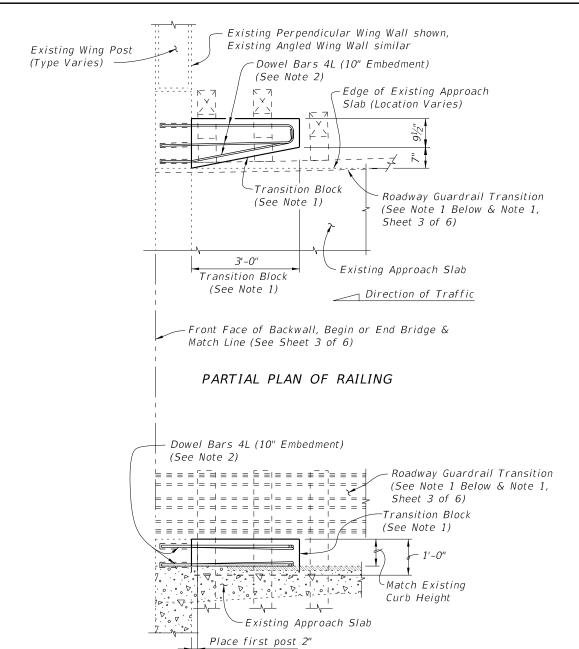
GUARDRAIL TRANSITIONS - EXISTING POST & BEAM BRIDGE RAILINGS (WIDE CURBS) INDEX

SHEET 2 of 6

*521-405* 

PARTIAL PLAN OF RAILING (SKEW ANGLE Ø = 70° OR GREATER)





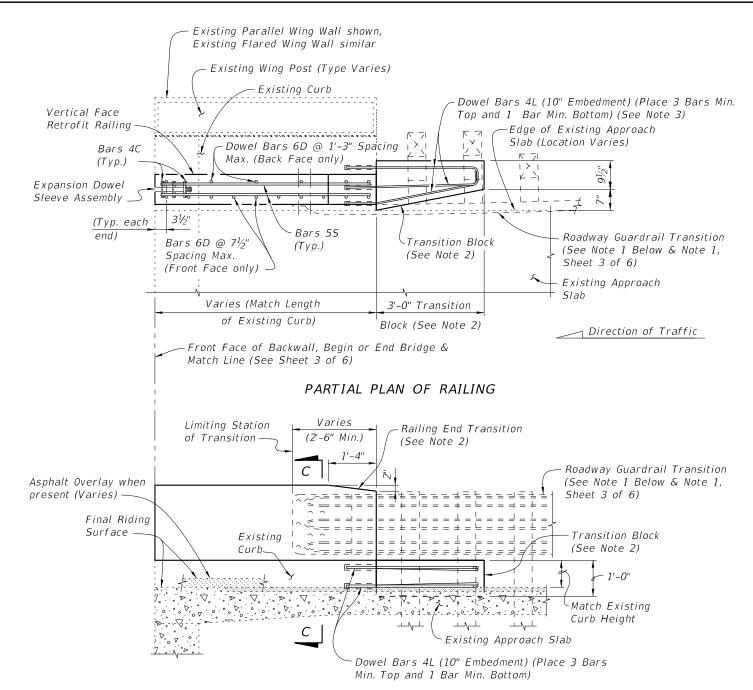
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

*≡ SCHEME 1 ≡* RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

clear of Wing Wall

## SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> ====== SCHEME 2 === RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 3 of 6. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

**REVISION** 07/01/13

DESCRIPTION:

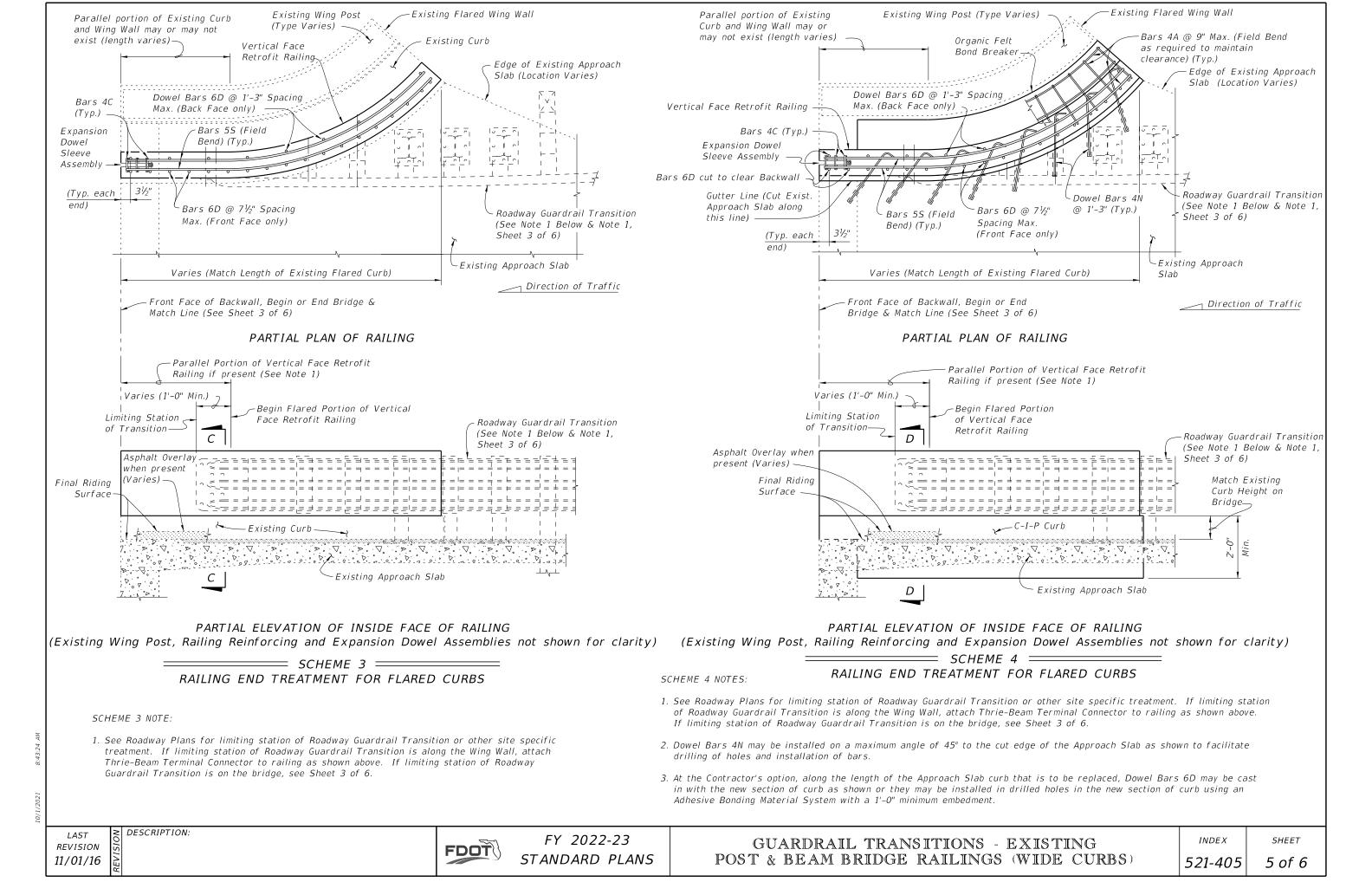
FDOT

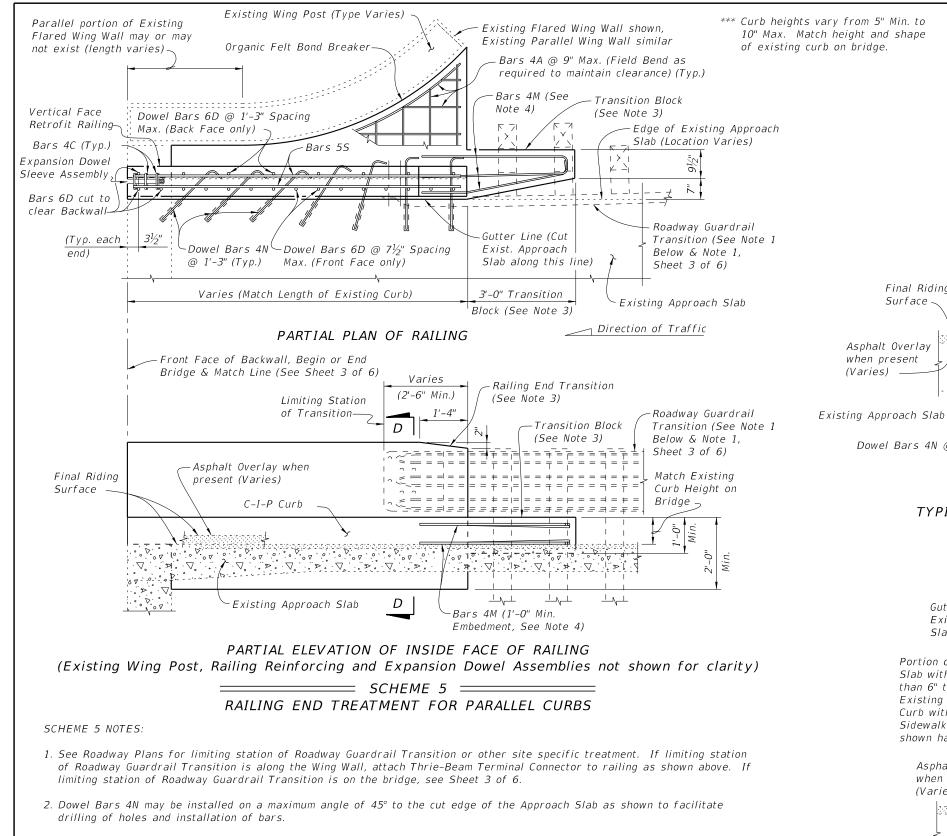
FY 2022-23 STANDARD PLANS

GUARDRAIL TRANSITIONS - EXISTING POST & BEAM BRIDGE RAILINGS (WIDE CURBS) *INDEX* 

SHEET 4 of 6

*521-405* 





- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment

(SCHEMES 4 AND 5 ONLY) GUARDRAIL TRANSITIONS - EXISTING

**REVISION** 11/01/16

DESCRIPTION:

FY 2022-23 STANDARD PLANS

INDEX SHEET POST & BEAM BRIDGE RAILINGS (WIDE CURBS) 6 of 6 *521-405* 

Wing Wall

TYPICAL SECTION THRU EXISTING APPROACH

SLAB AND END BENT WING WALL SHOWING LIMITS OF REMOVAL

Varies

2" Clear

Bars 55

Dowel Bars 6D

(See Note 5)

(Typ.)

(Typ.)

Existing Wing Post

(Type Varies)

Organic Felt

Bond Breaker

Bars 4A @ 9" Max

2" Min. Clear. Top

and Sides, 4" Min.

Clear. Bottom -

Bars 4A @ 9" Max., Min. 3 full length bars

required Top & Bottom (Field Bend to clear) (Typ.)

Varies (1'-2" Min.)

Guardrail Bolts

Varies

1" Min.

Final Riding

Dowel Bars 4N @ 1'-3" (Typ.)

Gutter Line (Cut

Existing Approach

Portion of Existing Approach

Slab with Integral Curb less than 6" thick or portion of

Existing Approach Slab and

Curb with Floating Detached Sidewalk to be removed

shown hatched.

(Varies)

Existing Approach Slab

Asphalt Overlay when present

Slab along this line) \_

Surface

Asphalt Overlay

when present

(Varies)

3" (Max.) Preferred

@ 111 (3¾"

'Embedmen

SECTION D-D

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

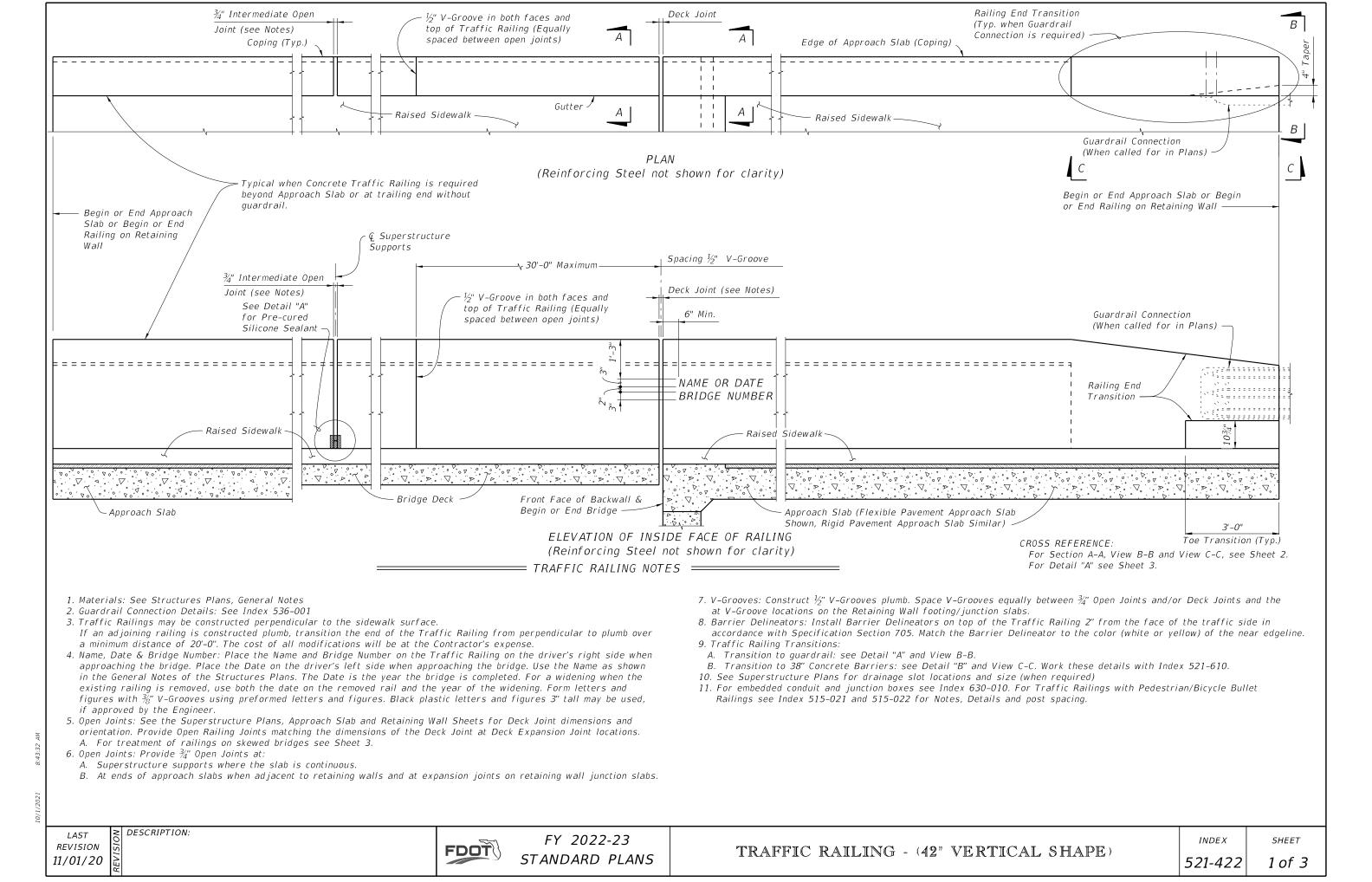
Existing Wing Post

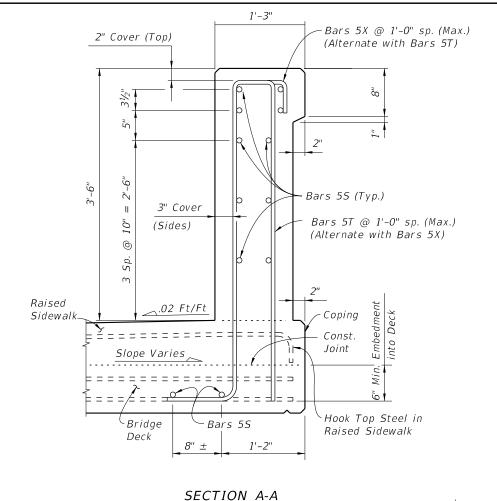
Varies (1'-2" Min.)

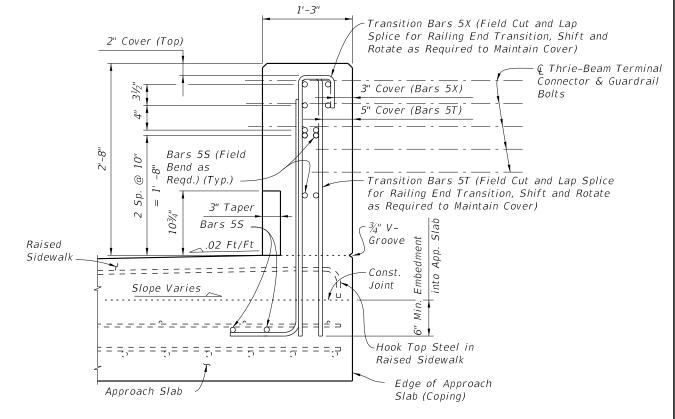
(Type Varies)

Varies

sp.





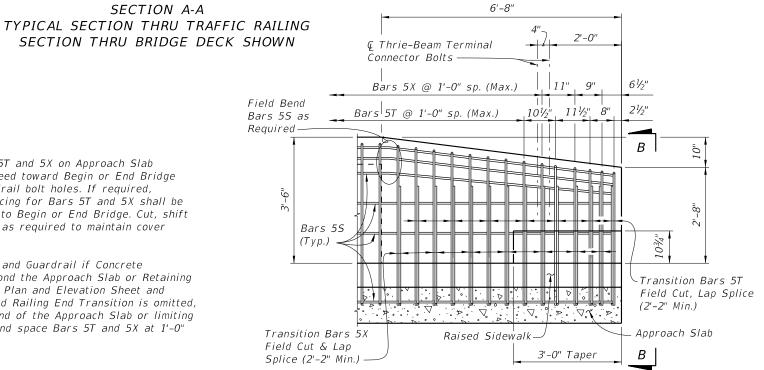


CROSS REFERENCE:

For location of Section A-A, View B-B

and View C-C, see Sheet 1.

VIEW B-B (END VIEW OF TRAFFIC RAILING END TRANSITION) (Approach Slab shown, Retaining Wall Junction Slab similar)



VIEW C-C RAILING END TRANSITION (Guardrail Not Shown For Clarity)

## NOTES:

- 1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.
- 2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)

**REVISION** 11/01/17

DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (42" VERTICAL SHAPE)

INDEX *521-422* 

SHEET

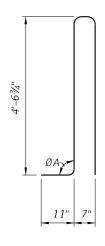
## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

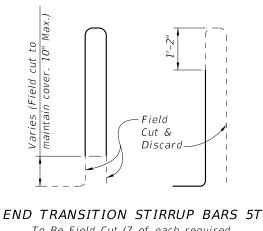
BILL OF REINFORCING STEEL			
MARK	SIZE	LENGTH	
S	5	As Reqd.	
Т	5	10'-8"	
Χ	5	6'-9"	

ROADWAY	Ø	А
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER
0% to 2%	90°	90°
2% to 6%	87°	83°
6% to 10%	84°	96°

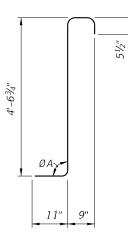


BAR 5S



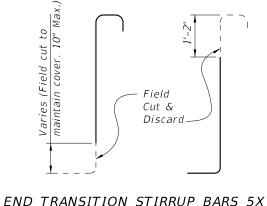






STIRRUP BAR 5X



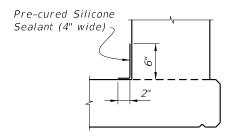


To Be Field Cut (7 of each required per Railing End Transition)

STIRRUP BAR 5T

#### REINFORCING STEEL NOTES:

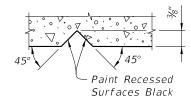
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The  $4'-6^3/4''$  vertical dimension shown for Bars 5T and 5X is based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slope vary from the above amounts, adjust this dimension accordingly to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above with  $\emptyset A = 90^{\circ}$ .
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTES:

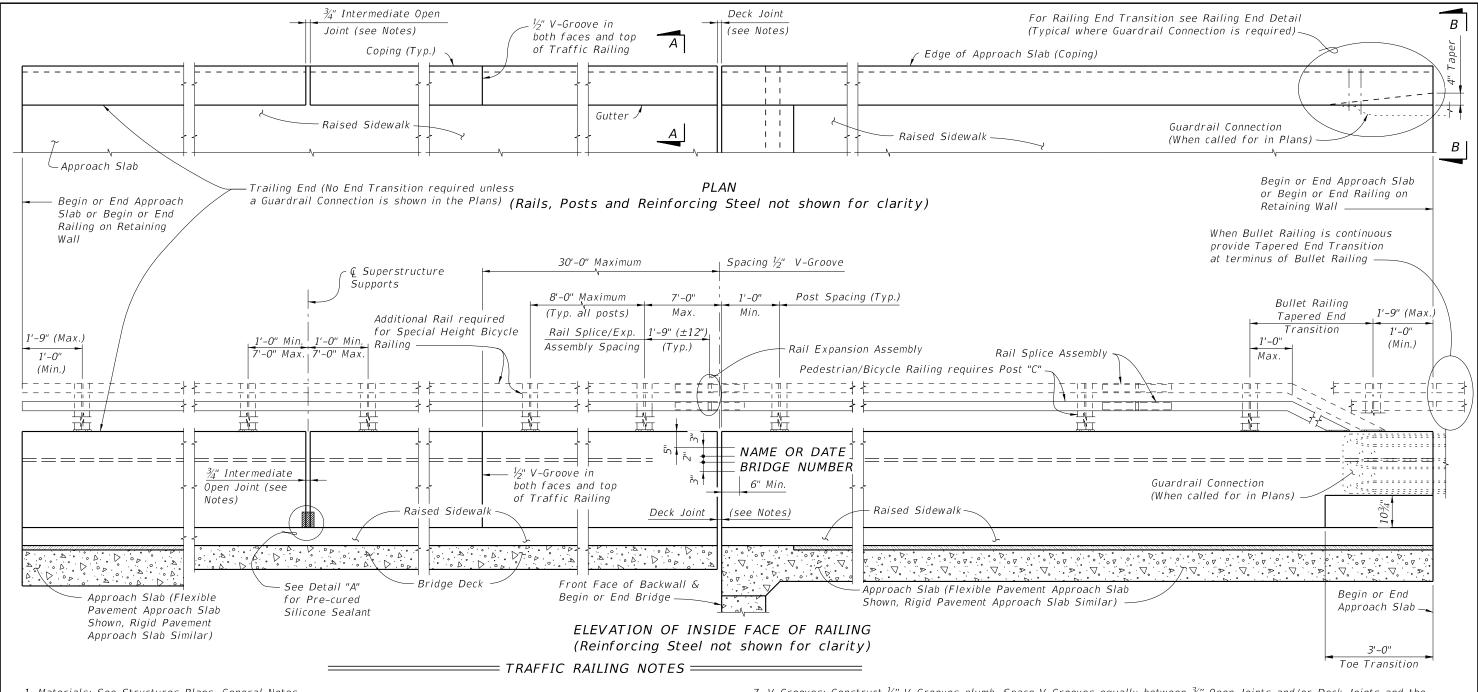
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



## SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.145	
Reinforcing Steel	LB/LF	30.68	

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)



- 1. Materials: See Structures Plans, General Notes
- 2. Guardrail Connection Details: See Index 536-001
- 3. Traffic Railings may be constructed perpendicular to the sidewalk surface. If an adjoining railing is constructed plumb, transition the end of the Traffic Railing from perpendicular to plumb over a minimum distance of 20'-0". The cost of all modifications will be at the Contractor's expense.
- 4. Name, Date & Bridge Number: Place the Name and Bridge Number on the Traffic Railing on the driver's right side when approaching the bridge. Place the Date on the driver's left side when approaching the bridge. Use the Name as shown in the General Notes of the Structures Plans. The Date is the year the bridge is completed. For a widening when the existing railing is removed, use both the date on the removed rail and the year of the widening. Form letters and figures with  $rac{3}{2}$ " V-Grooves using preformed letters and figures. Black plastic letters and figures 3" tall may be used, if approved by the Engineer.
- 5. Open Joints: See the Superstructure Plans, Approach Slab and Retaining Wall Sheets for Deck Joint dimensions and orientation. Provide Open Railing Joints matching the dimensions of the Deck Joint at Deck Expansion Joint locations. A. For treatment of railings on skewed bridges see Sheet 3.
- 6. Open Joints: Provide 3/4" Open Joints at:

DESCRIPTION:

- A. Superstructure supports where the slab is continuous.
- B. At ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.

- 7. V-Grooves: Construct 1/2" V-Grooves plumb. Space V-Grooves equally between 1/4" Open Joints and/or Deck Joints and the at V-Groove locations on the Retaining Wall footing/junction slabs.
- 8. Barrier Delineators: Install Barrier Delineators on top of the Traffic Railing 2" from the face of the traffic side in accordance with Specification Section 705. Match the Barrier Delineator to the color (white or yellow) of the near edgeline. 9. For embedded conduit and junction boxes see Index 630-010.
- 10. For Traffic Railings with Pedestrian/Bicycle Bullet Railings see Index 515-021 and 515-022 for Notes, Details and post spacing.

CROSS REFERENCE: For Section A-A and View B-B, see Sheet 2. For Detail "A" see Sheet 3.

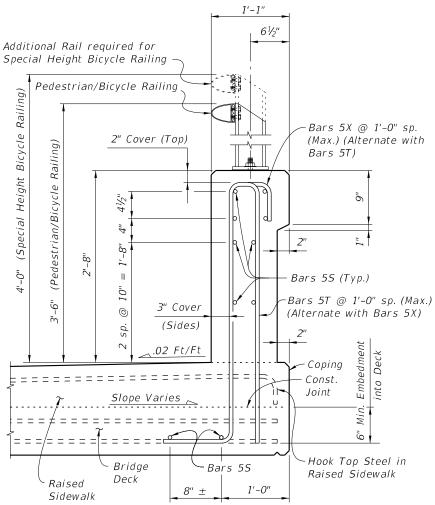
REVISION 11/01/20

**FDOT** 

FY 2022-23 STANDARD PLANS

INDEX *521-423* 

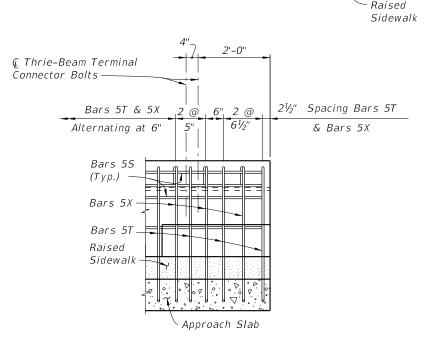
SHEET 1 of 3



## SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING (Section Thru Bridge Deck shown)

#### NOTES:

- 1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.
- 2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)



RAILING END DETAIL (Guardrail Not Shown For Clarity)

**REVISION** 11/01/17

DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (32" VERTICAL SHAPE)

1'-1"

- Bars 5X @ 1'-0" sp. (Max.) (Alternate with

Const. Joint

Bars 5S

1'-0"

CROSS REFERENCE:

see Index 515-022.

see Sheet 1.

VIEW B-B

APPROACH SLAB END VIEW

OF TRAFFIC RAILING

Bars 5T) (See Note 1)

Hook Top Steel in

Edge of Approach

Raised Sidewalk

Slab (Coping)

For location of Section A-A and View B-B

NOTE: For Bullet Railing Details,

♀ Thrie-Beam Terminal

Connector & Guardrail

Bolts

Bars 5S (Field Bend as

Bars 5T @ 1'-0" sp. (Max.)

(Alternate with Bars 5X)

Required) (Typ.)

(See Note 1)

Additional Rail required for

Railing)

Bicycle

Raili

(Pedestrian/Bicycle

Special Height Bicycle Railing

Pedestrian/Bicycle Railing

2" Cover (Top)

3" Taper

.02 Ft/Ft

Slope Varies

Approach

Slab

INDEX

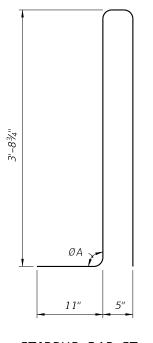
SHEET

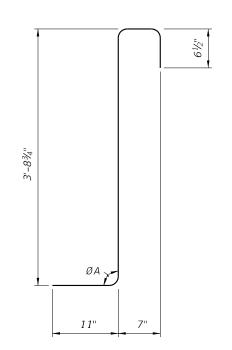
*521-423* 2 of 3

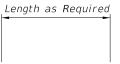
## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL			
MARK	SIZE	LENGTH	
5	5	As Reqd.	
Т	5	9'-0"	
Х	5	5'-10"	

ROADWAY	Ø	A
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER
0% to 2%	90°	90°
2% to 6%	87°	93°
6% to 10%	84°	96°







BAR 5S

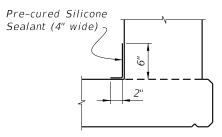
STIRRUP BAR 5T

DESCRIPTION:

STIRRUP BAR 5X

## REINFORCING STEEL NOTES:

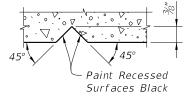
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 3'-8¾" vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into
- 3. The reinforcement for the railing on a Retaining Wall shall be the same as detailed with  $\emptyset A = 90^{\circ}$ .
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTES:

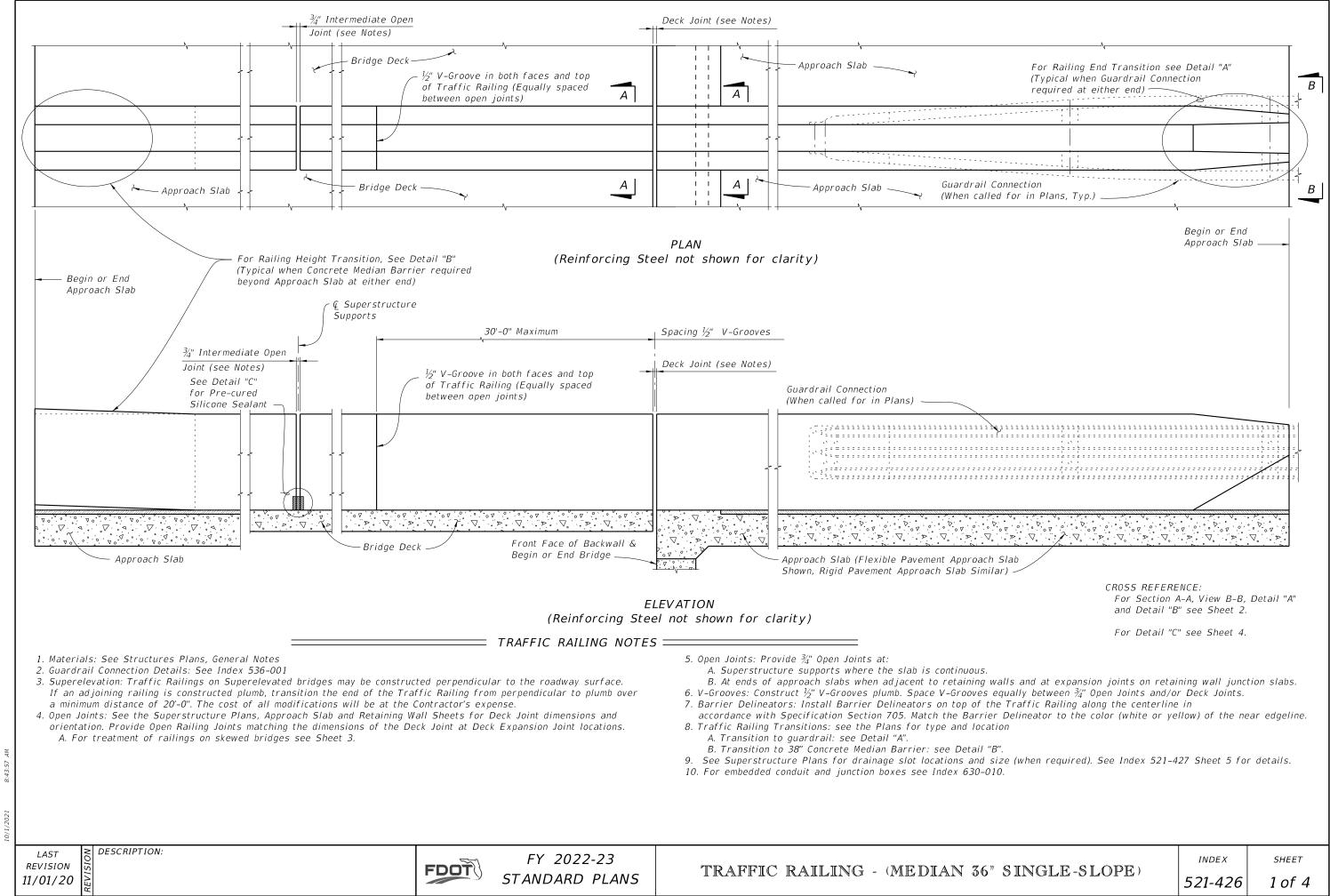
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.

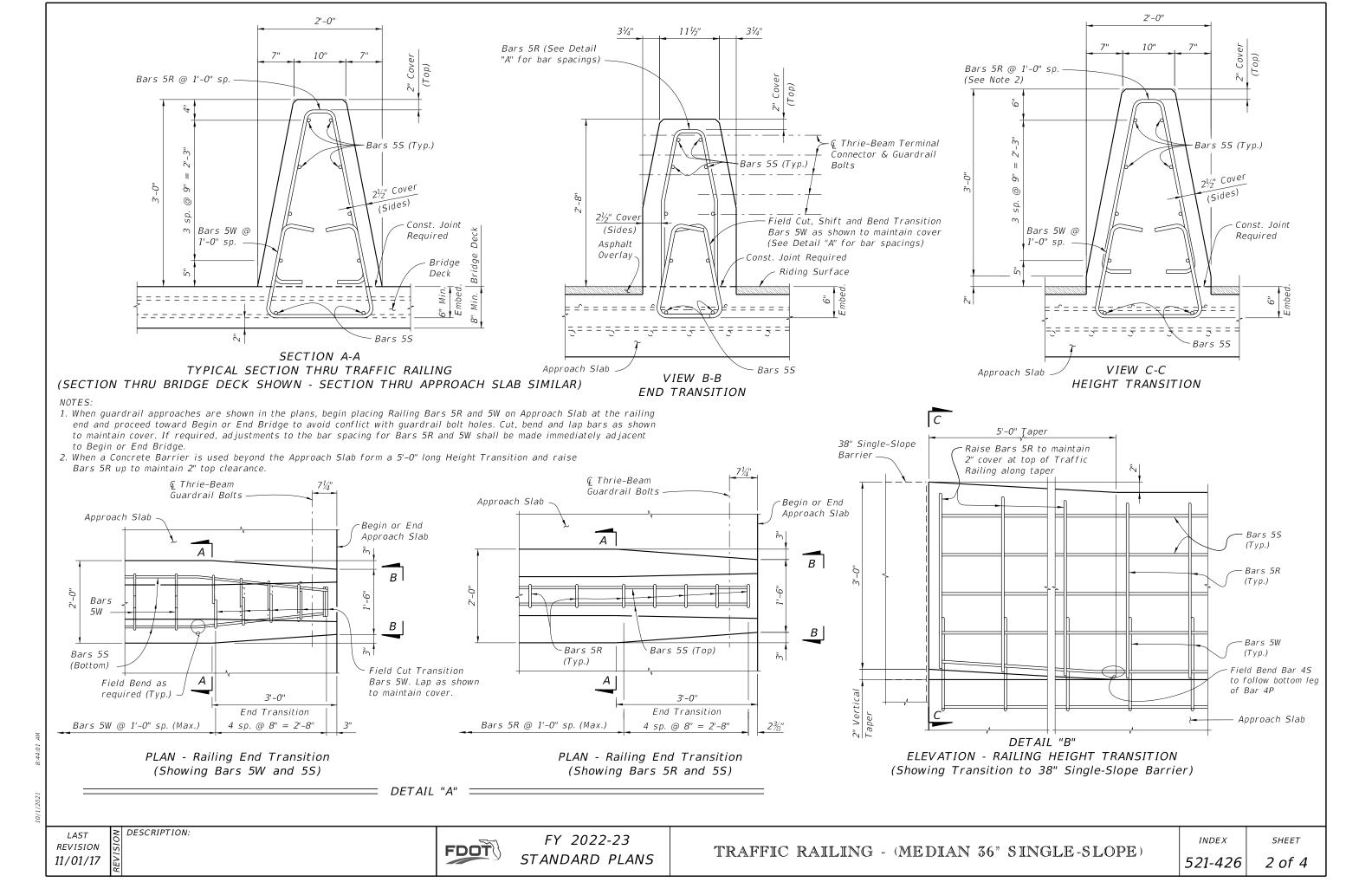


## SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED : QUA	TRAFFIC I NTITIES	RAILING
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.095
Reinforcing Steel	LB/LF	25.90

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope.)



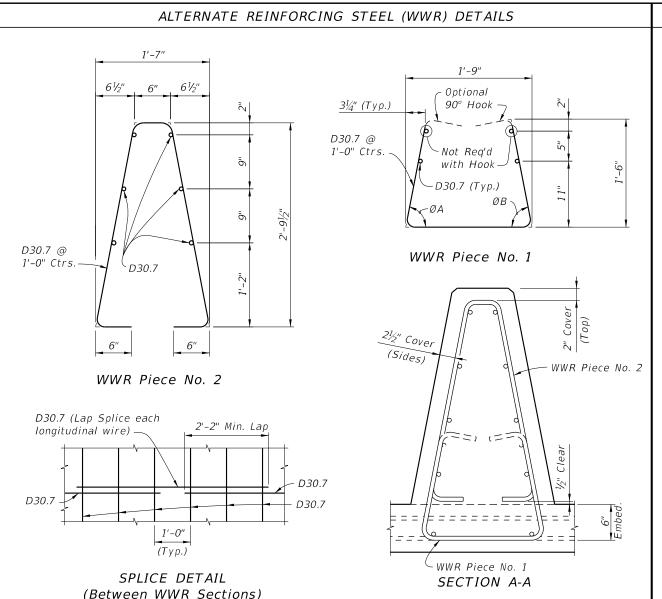


# PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

#### NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4) ¾" Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the Ç of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

DESCRIPTION:



#### **ROADWAY** ON SLOPE AT CROWN CROSS-SLOPE ØΑ ØВ ØΑ ØВ 7*9*° 7*9°* 79° 79° 0% to 2% >2% to 6% 81° 77° 79° 79° 79° $84^{\circ}$ 74° 79° >6% to 10%

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

ØA and ØB shall be 79° if Contractor elects to place railing perpendicular to the deck, and approach slabs.

BILL OF REINFORCING STEEL			
MARK SIZE LENGTH			
R	5	7'-2"	
5	5	As Reqd.	
W	5	5'-10"	

Length as Required

BAR 5S

1'-7" 61/2" 6" 61/2" Field Bend as required to maintain cover  $D = 2\frac{1}{2}$ " - Field Cut & Discard 6"

1'-2" Field Cut & / Reuse -ØA or ØB to match Typ. Bars Optional Splice (see Note 4) 10" 1'-9"

STIRRUP BAR 5R

TRANSITION STIRRUP BAR 5R (5 required per Railing End Transition)

STIRRUP BAR 5W

TRANSITION STIRRUP BAR 5W To Be Field Cut (10 required per Railing End Transition)

#### WELDED WIRE REINFORCEMENT NOTES:

DESCRIPTION:

- 1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 5R, 5S and 5W. WWR must meet the requirements of Specification Section 931.
- 2. WWR at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The bottom of Piece 1 shall be cut to allow overlap.
- 3. Place WWR panels so as to minimize the end overhang of longitudinal wires at Railing Ends and Open Joints. Overhangs greater than 6" are not permitted.

#### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 4. At the Contractor's option, Bars 5W may be fabricated as a two piece bar with a 1'-2" lap splice of the bottom legs.

# Pre-cured Silicone Sealant 4" wide (Typ.) (Typ.)

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM UNIT QUANTITY			
Concrete	CY/LF	0.157	
Reinforcing Steel	LB/LF	23.99	

(The above quantities are based on a crowned roadway, with a 2% cross slope)

REVISION 01/01/18

**FDOT** 

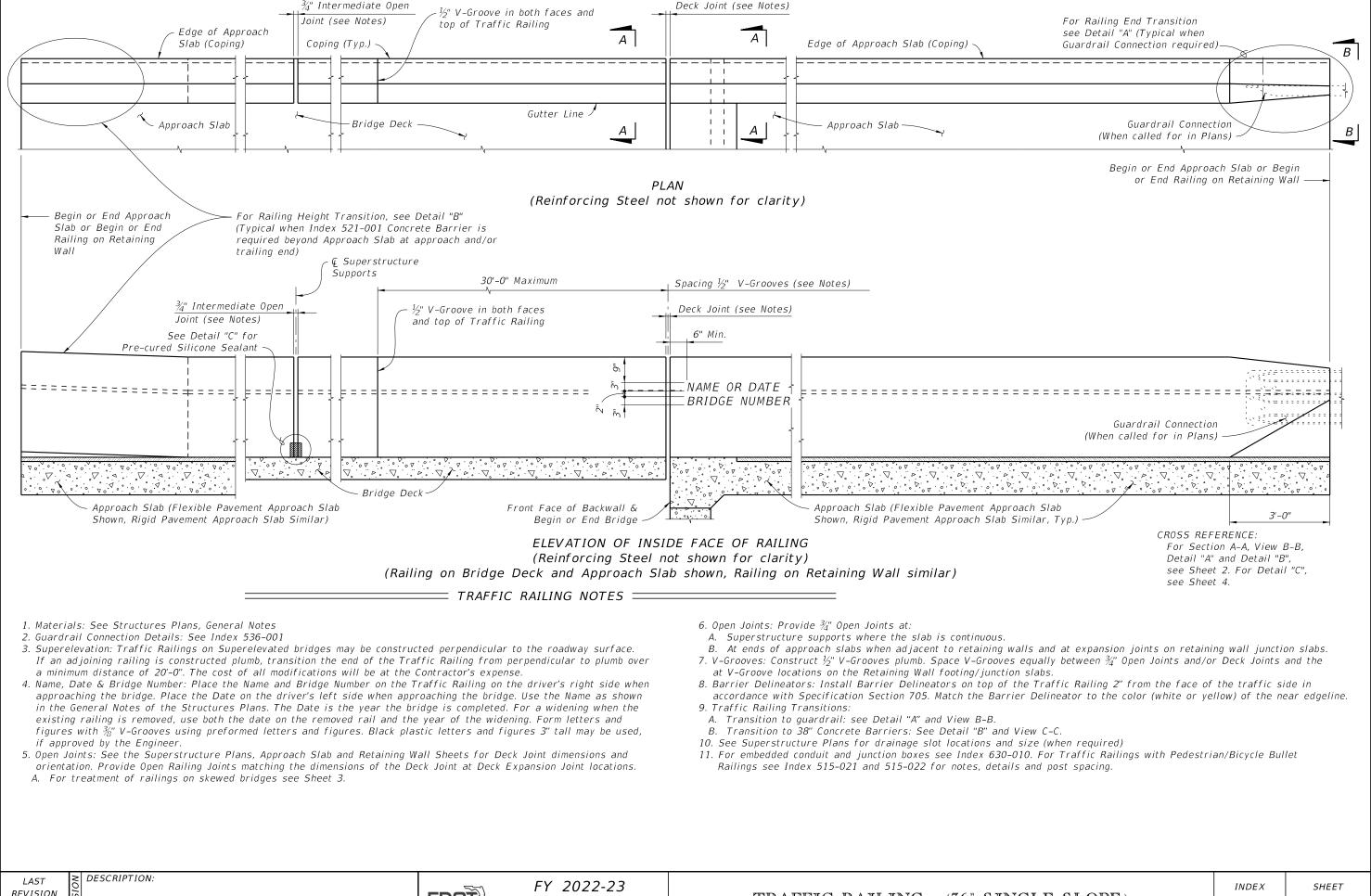
FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (MEDIAN 36" SINGLE-SLOPE)

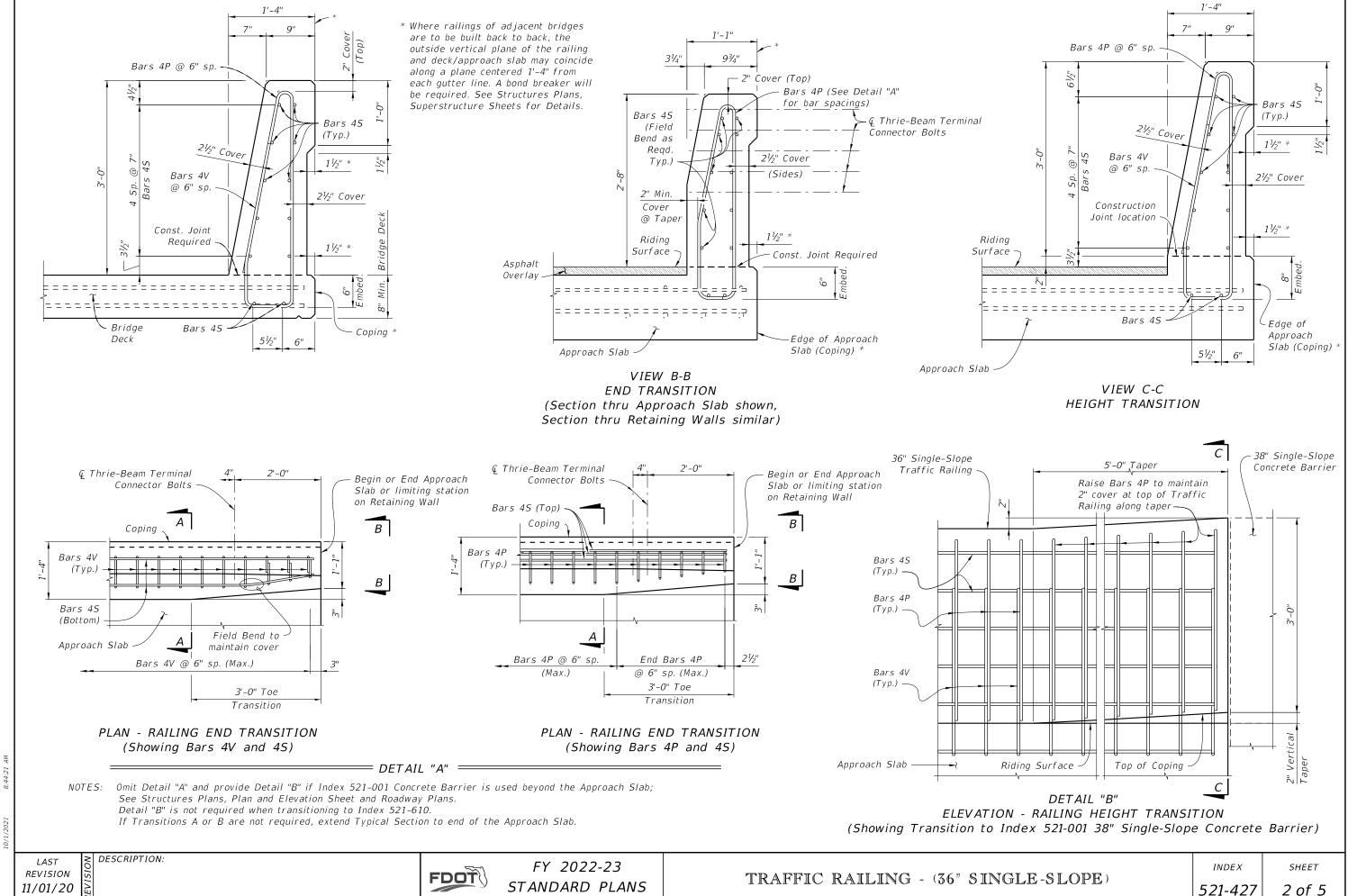
INDEX *521-426* 

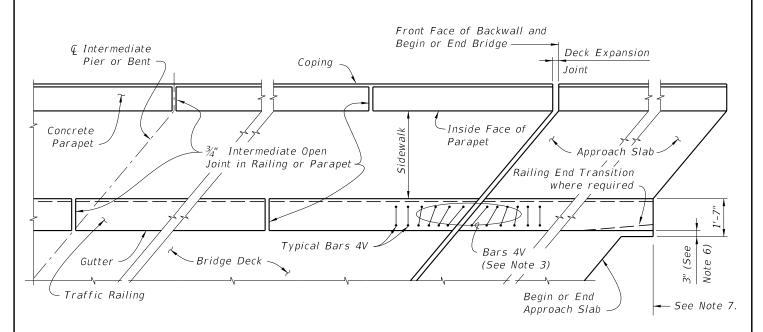
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4 of 4



REVISION 11/01/20

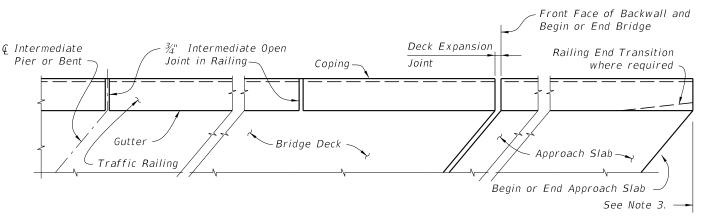




PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK, SINGLE-SLOPE TRAFFIC RAILING AND PEDESTRIAN/BICYCLE RAILING INDEX 521-820 or 521-825, OTHER TRAFFIC RAILINGS SIMILAR

#### NOTES:

- 1) Concrete Parapet reinforcement is not effected by skew angle, see Index 521-820 for details.
- 2) Parapet expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 3) Traffic Railing reinforcement vertical Bars 4V & 4P may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement. Bars 4V adjacent to expansion joints shall be field adjusted to maintain clearance and spacing, extra Bars 4V will be required. Cut bottom horizontal portion of 4V Bars to maintain maximum horizontal length to each vertical leg being placed. Discard the remainder of the bar. Rotate cut bars to maintain clearance.
- 4) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 5) 3/4" Intermediate Open Joints and V-Grooves in railing and parapet shall be placed perpendicular or radial to the gutter line or inside face of parapet line. See Structures Plans, Superstructure Sheets for locations.
- 6) At begin or end approach slab extend slab at the railing ends 3" (gutter side or back face of railing as required) as shown to provide a base for casting of the railing. Field trim toe of Bars 4V by 1 inch as required to maintain concrete cover at edge of deck.
- 7) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.



# PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SINGLE-SLOPE TRAFFIC RAILING, OTHER TRAFFIC RAILINGS SIMILAR

#### NOTES:

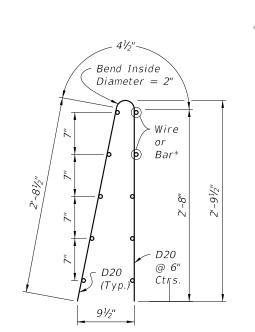
- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 2) ¾" Intermediate Open Joints and ½" V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 3) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.

GENERAL NOTES:

- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes as applicable.
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing.
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

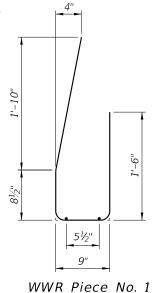
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DESCRIPTION:

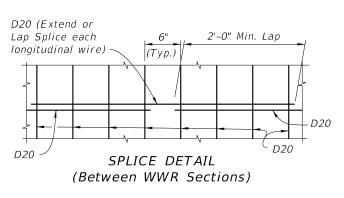


\*Longitudinal D20 Wires or #4 Bars may be tied.

ALTERNATE REINFORCING STEEL (WWR) DETAILS



## WWR Piece No. 2



WWR Piece No. 2 21/2" Cover 21/2" Cover ➤ WWR Piece No. 1

#### WELDED WIRE REINFORCEMENT NOTES:

- 1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 4P, 4S and 4V. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.
- 2. WWR at Railing End Transition shall be field bent inward as required (Piece 2) to maintain cover. The bottom of the vertical wires (D20) in Piece 2 shall be cut a maximum of 4 inches and the gutter side portion bent inward as required to allow placement.

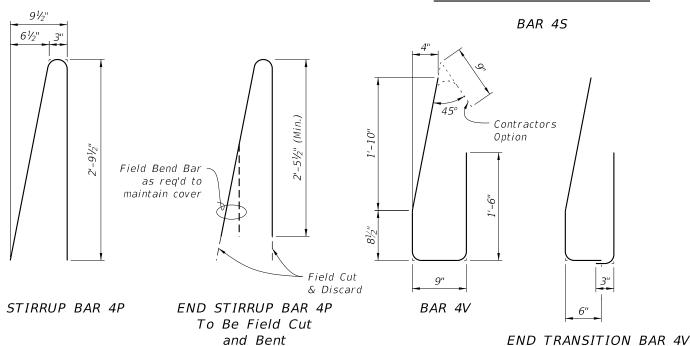
## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

ROADWAY	LOW GUTTER	HIGH GUTTER
CROSS-SLOPE	ØB	ØB
0% to 2%	90°	90°
2% to 6%	87°	93°
6% to 10%	84°	96°

ØB shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.

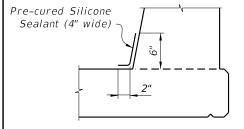
BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
Р	4	5'-11"		
5	4	As Reqd.		
V	4	4'-10"		

Length as Required



REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The  $8lac{1}{2}$ " vertical dimensions shown for Bar 4V is based on a 6" embedment into the bridge deck without a raised sidewalk. If a raised sidewalk is to be provided, increase this dimension to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 2'-0".

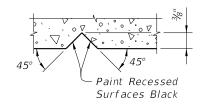


DESCRIPTION:

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM	UNIT	QUANTITY		
Concrete	CY/LF	0.107		
Reinforcing Steel	LB/LF	24.78		

(The above quantities are based on a 2% deck cross slope; railing on low side of deck.)

REVISION 11/01/17

**FDOT** 

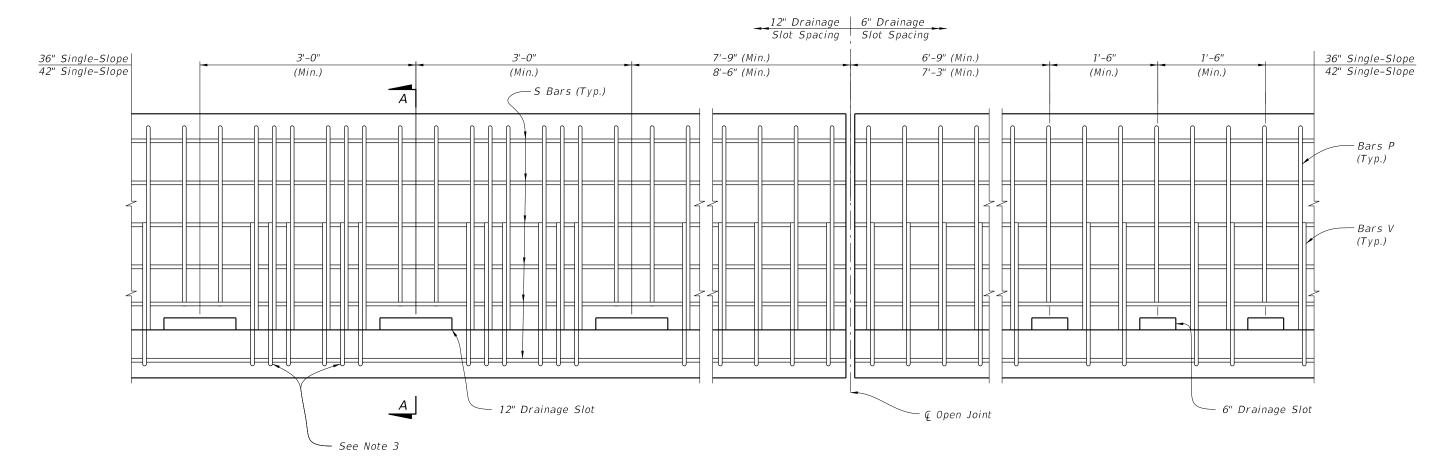
FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (36" SINGLE-SLOPE)

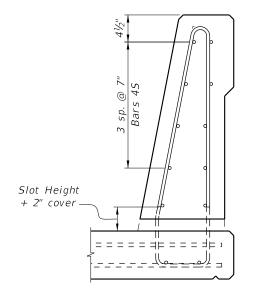
INDEX

SHEET

Field Cut and Lapped







SECTION A-A 36" Single-Slope Shown Other traffic railings similar

## DRAINAGE SLOT NOTES:

- 1. Use only when required for safety. See Plans for locations and size of drainage slots.
- 2. Maintain 2" minimum cover to all reinforcing. Trim P Bars over drainage slots and raise bottom S bars as necessary to maintain cover.
- 3. For slots greater than 6" in length, add additional vertical bars (V & P) on each side of the opening.
- 4. Drainage slot heights are 2" or 3". See the plans for size and location details.

REVISION 11/01/19

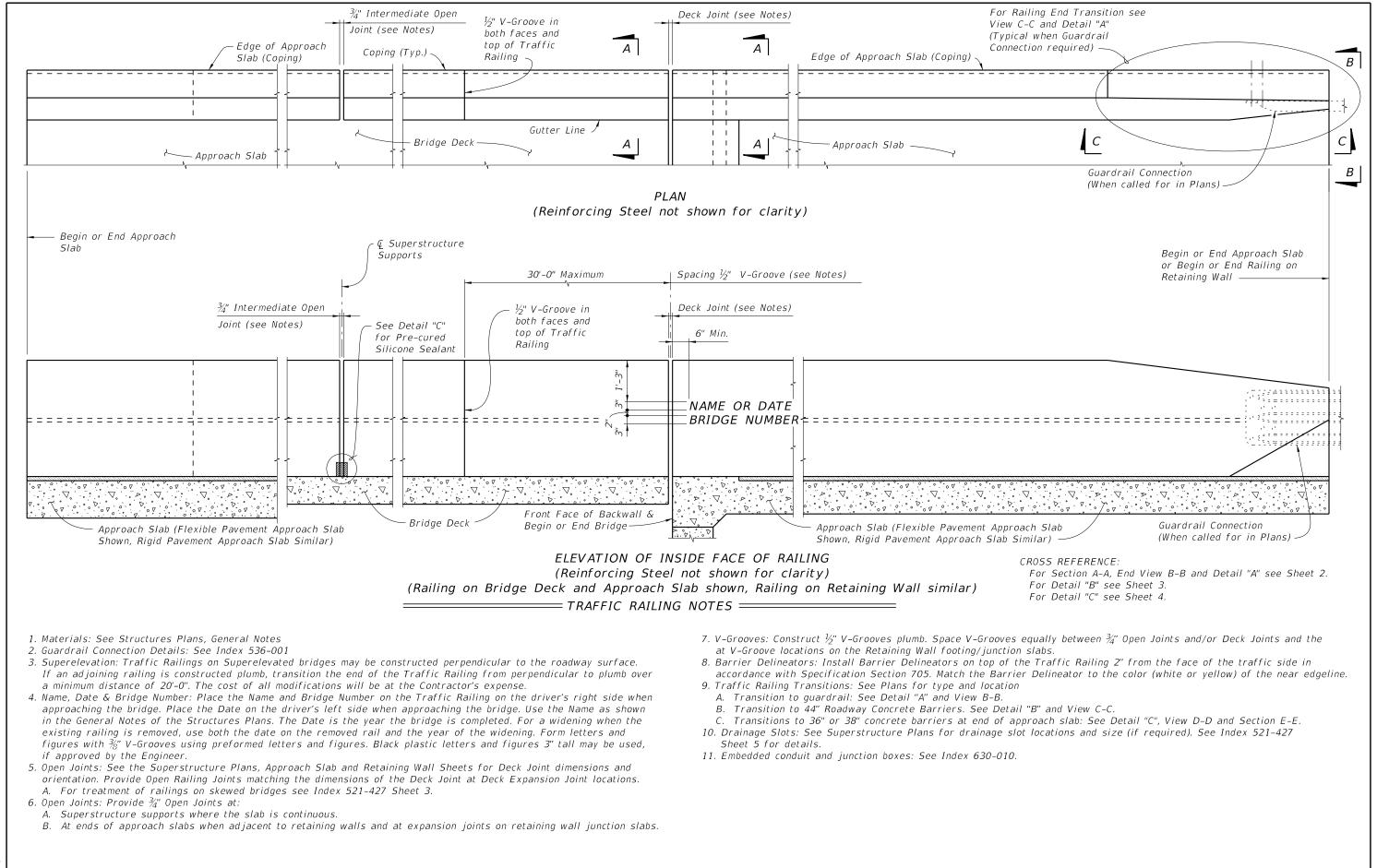
DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

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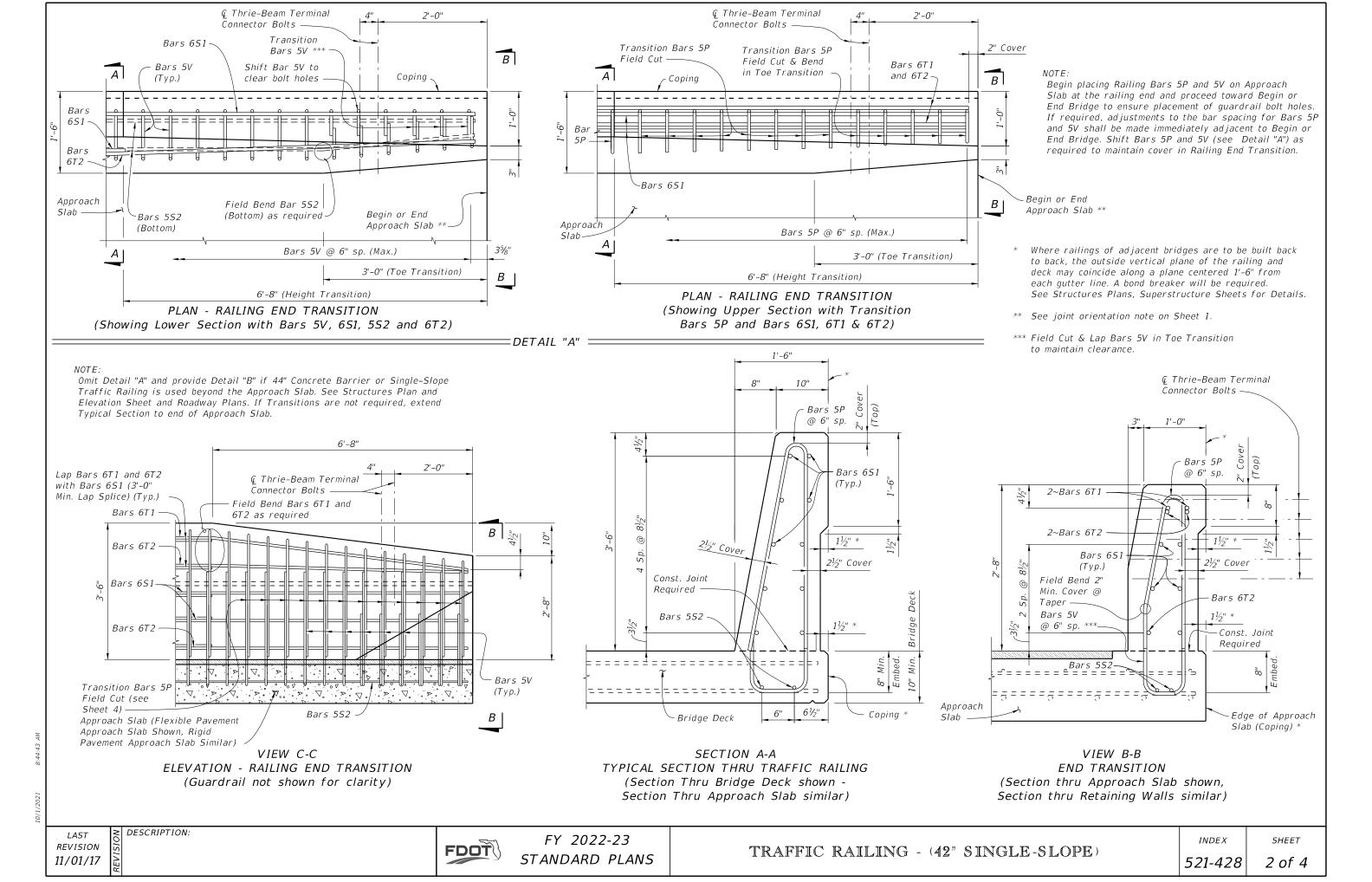
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LAST CONTREVISION TO 11/01/20

DESCRIPTION:

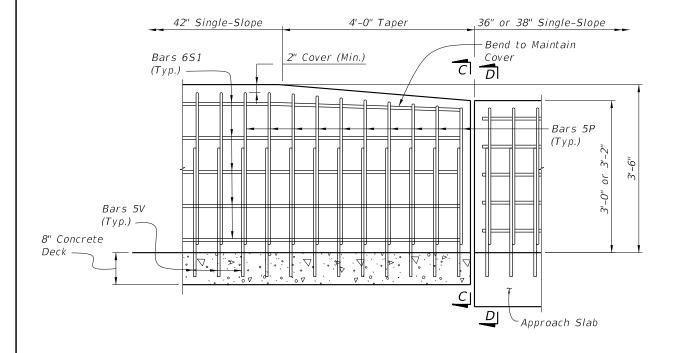
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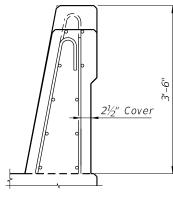
SHEET



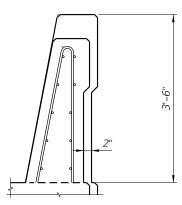
- 1. Provide Detail "B" height transition where 42" Traffic Railings are required on bridge, and 36" or 38" Barriers are shown on approaches. See Structures Plans for coping details.

  2. Work Detail "B" with Indexes 400-090 or 400-091, 521-427,
- and 521-610 as necessary.
- 3. Field cut 5P Bars as shown to maintain 2" min. (4" max.) cover at top of traffic railing.





VIEW C-C RAILING HEIGHT TRANSITION (Begin/End of Bridge) (Bars 5V not shown for clarity)



SECTION D-D (Index 400-091 Shown, 400-090 Similar) (Index 521-427 Bars 4V not shown for Clarity)

DETAIL "B"

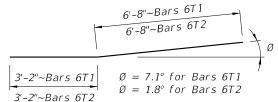
≥ DESCRIPTION:

## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
Р	5	7'-0"		
S1	6	As Reqd.		
52	5	As Reqd.		
T1 & T2	6	10'-0"		
V	5	5'-9"		

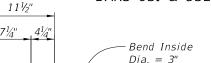
ROADWAY	LOW GUTTER	HIGH GUTTER
CROSS-SLOPE	ØВ	ØВ
0% to 2%	101°	101°
2% to 6%	98°	104°
6% to 10%	95°	107°

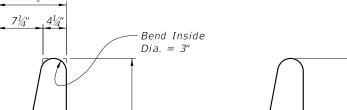
ØA and ØB shall be 90° if Contractor elects to place Railing perpendicular to the Deck.



Length as Required

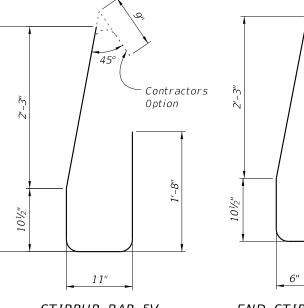
BARS 651 & 552





Field Bend -(as required to maintain cover) Field Cut &

TRANSITION BARS 6T1 & 6T2 (2~Bars 6T1 & 3~Bars 6T2 required per Railing End Transition)



STIRRUP BAR 5P

TRANSITION STIRRUP BAR 5P To Be Field Cut (10 of each required per Railing End Transition)

Discard

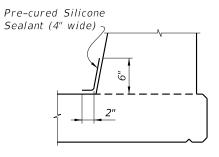
STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut and Lapped

REINFORCING STEEL NOTES:

DESCRIPTION:

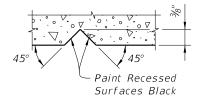
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 6S1 may be continuous or spliced at the construction joints. Lap splices for Bars 6S1 and 5S2 shall be a minimum of 3'-0" and 2'-2", respectively.
- 4. The Contractor may utilize deformed WWR when approved by the Engineer. WWR must meet the requirements of Specification Section 931.



DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

#### INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



## SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.143	
Reinforcing Steel	LB/LF	39.34	

The estimated railing quantities are based on a 2% deck cross slope; railing on low side of deck.

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

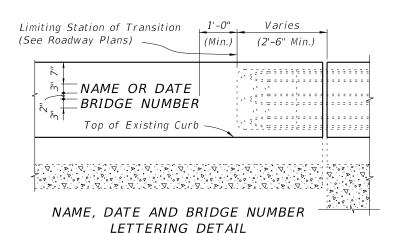
ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment). BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments.

Details for bridges on horizontally curved alignments are similar.

NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or  $\frac{3}{6}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures. ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

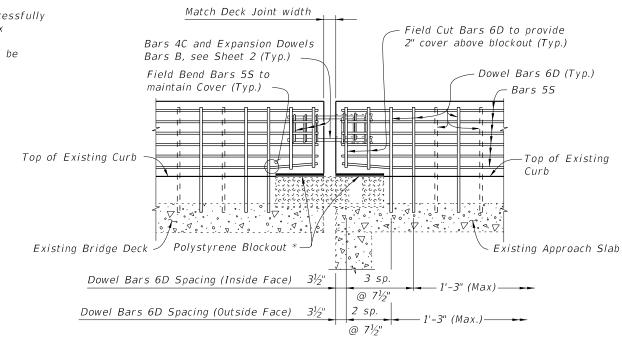
BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing 2" from the face on the traffic side in accordance with Specification Section 705. Match the Barrier Delineator color (white or yellow) to the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) includes all materials and labor required to construct the railing and incidental work as required for transition blocks, curbs, spread footing approaches, and Barrier Delineators.



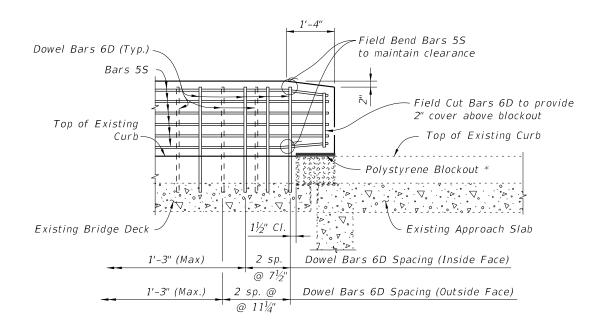
ESTIMATED TRAFFIC RAILING QUANTITIES				
QUANTITY		ITITY		
UNIT	9" Curb	Increment		
CY/FT	0.064	0.003 per in. height		
LB/FT	13.27	0.10 per in. length		
	UNIT CY/FT	UNIT  QUAN  9" Curb  CY/FT 0.064		

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index 521-484, Sheet 4 for Spread Footing Approach Quantities.



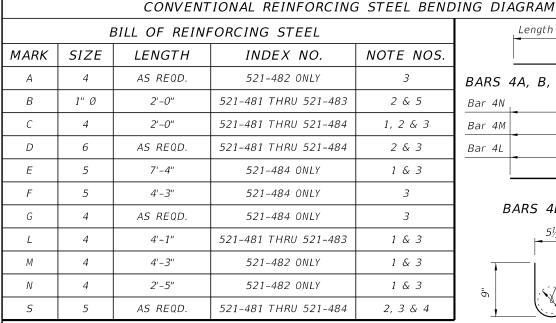
PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT - SCHEMES 2 THRU 5 (Begin or End Bridge Shown, Intermediate Joints Similar)

\* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEME 1 (Guardrail Transition not shown for clarity)

1 of 2 *521-480* 



1. All bar dimensions in the bending diagrams are out to out.

4. Bars 5S may be continuous or spliced at the construction

5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the

the same as detailed for a bridge deck.

2. The reinforcement for the railing on a retaining wall shall be

3. All reinforcing steel in the Vertical Face Retrofit Railing shall

joints. Bar splices for Bars 5S shall be a minimum of 2'-2".

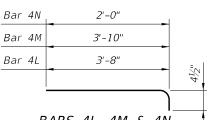
REINFORCING STEEL NOTES:

have a 2" minimum cover.

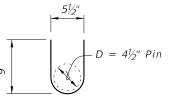
Specifications.

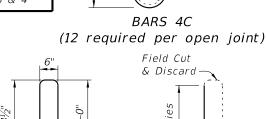
## Length as Required

BARS 4A, B, 6D, 5F, 4G & 5S



BARS 4L, 4M & 4N





BARS 5E BARS 5E (Typical Section) (Tapered End Transition)

OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)

1'-0" Expansion Sleeve Assembly

Spacing Pairs of Bars 4C

2½" (Shift Bars 4C

skewed joints)

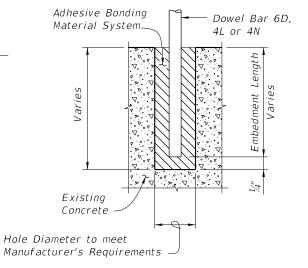
to clear Bars 6D for

Length of Expanded Polystyrene

Plug to match width of open joint

1" Ø PVC Pipe Sleeve

Cap & Polystyrene Plug



#### DOWEL DETAIL

Dowel Installation Notes:

- 1. Shift dowel holes to clear if the existing reinforcement is encountered.
- 2. See Index 521-481 thru 521-484 for required embedment length of Bars 6D, 4L or 4N.

\*  $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.

\*\* See Index 521-481 thru 521-484 for spacing of Bars 6D.

3/4" Int. Open Joint

or Deck Joint

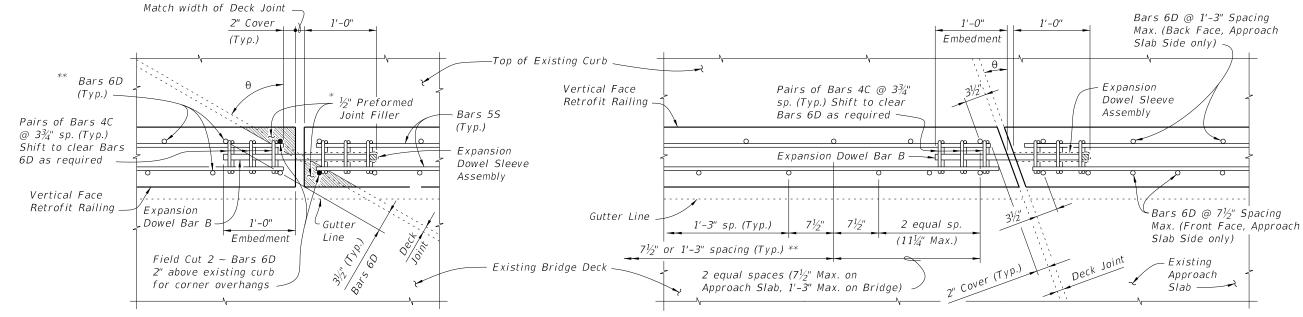
2 sp. @ 3¾"

Top of Existing

Curb

Spacing Expansion

Dowel Bars B



PARTIAL PLAN OF RAILING (SKEW ANGLE \theta GREATER THAN 20°) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

PARTIAL PLAN OF RAILING (SKEW ANGLE  $\theta = 20^{\circ}$  OR LESS) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

SKEW DETAIL

**REVISION** 11/01/16

DESCRIPTION:

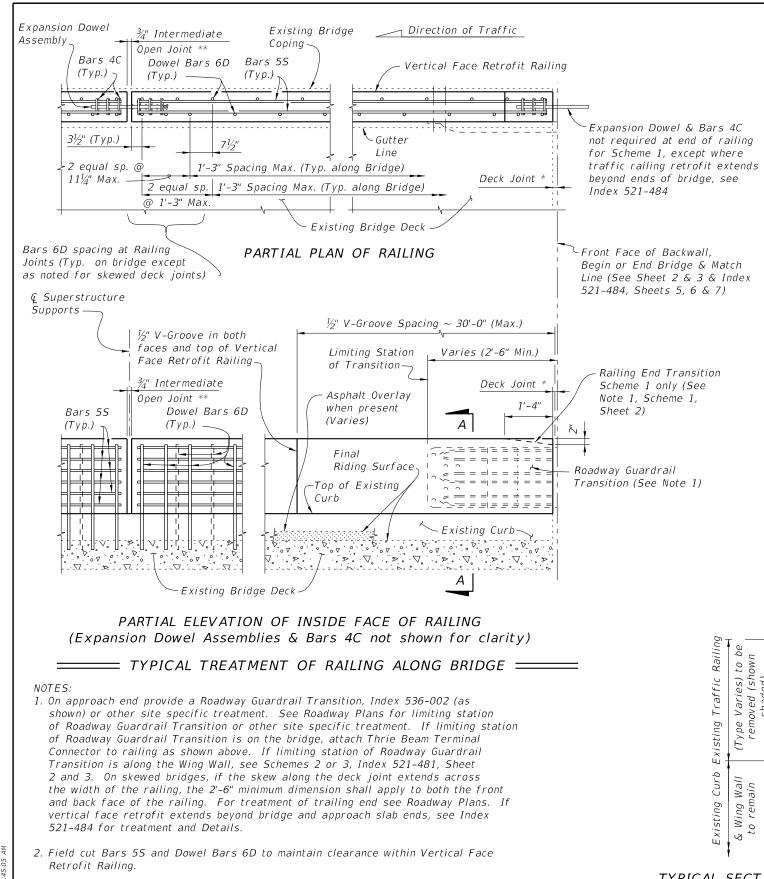
FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) TYPICAL DETAILS & NOTES

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3. Where existing structure has been removed and not encased in new concrete; match

adjoining areas and finish flat by grouting or grinding as required. Exposed existing

reinforcing steel not encased in new concrete shall be burned off 1" below existing

TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL

Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Varies Skew Detail, Index 521-480. Provide open Railing Joints at Deck Expansion Joint locations matching the dimension of Varies Varies (3" max. and Preferred, 10" | (1" Min.) 1" Min., constant for full length of Retrofit) ← Thrie-Beam
 ← Thrie-Be (1) - Superstructure supports where slab is continuous. Guardrail Bolts 2" Cover (Typ.)Bars 55 (Typ.)Dowel Bars 6D -Existing Asphalt Overlay Curb when present 6" Min. Embed Min. (Varies) \_

SECTION A-A TYPICAL SECTION THRU RAILING ON CURB WITH CORBELS

Existing Bridge Deck

#### CROSS REFERENCE:

the Deck Joint.

\*\* Provide 3/4" Intermediate Open Joints at:

\*\*\* Curb heights vary from 5" Min. to 1'-2" Max.

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagrams see Index 521-480.

Varies Varies (3" max. and Preferred, 10" Varies 1" Min., constant for full (1" Min.) *length of Retrofit)* ← Thrie-Beam Guardrail Bolts 2" Cover (Typ.)@ 17 (3¾" Bars 55 (Typ.)Dowel Bars 6D Existina Asphalt Overlay Curb when present (Varies) Min. Final Riding Surface Bridge Deck Existing Bridge Deck

SECTION A-A TYPICAL SECTION THRU RAILING ON FULL DEPTH CURB (BRIDGE SHOWN, WING WALL SIMILAR)

(BRIDGE DECK SHOWN, WING WALL SIMILAR)

REVISION 07/01/13

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) NARROW CURB

Final Riding Surface

Bridg

INDEX 521-481

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concrete and grouted over.

#### PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

Existing Perpendicular Wing Wall shown,

Dowel Bars 4L (10" Embedment)

-Edge of Existing Approach

Slab (Location Varies)

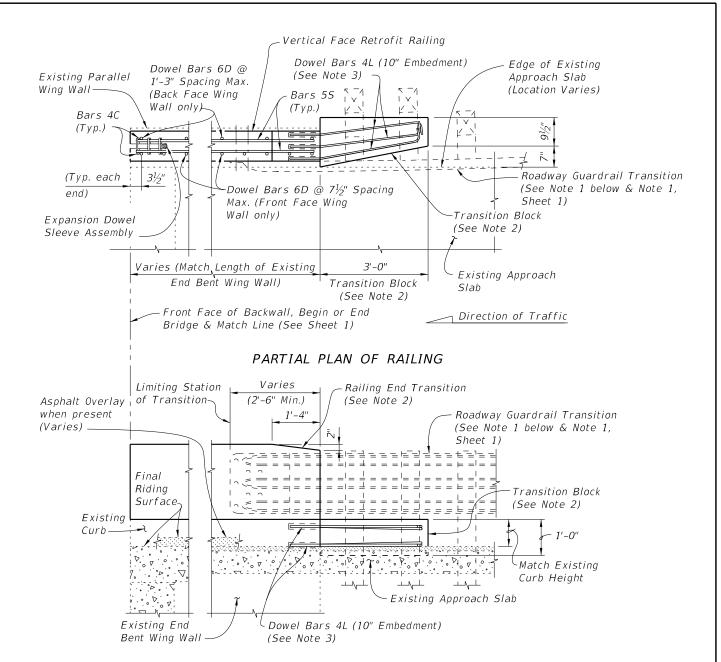
Existing Angled Wing Wall similar

(See Note 2)

## == SCHEME 1 ====== RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

#### SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

## \_\_\_\_ SCHEME 2 \_\_\_\_\_ RAILING END TREATMENT FOR PARALLEL WING WALLS

#### SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Index 521-481, Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

REVISION 07/01/07

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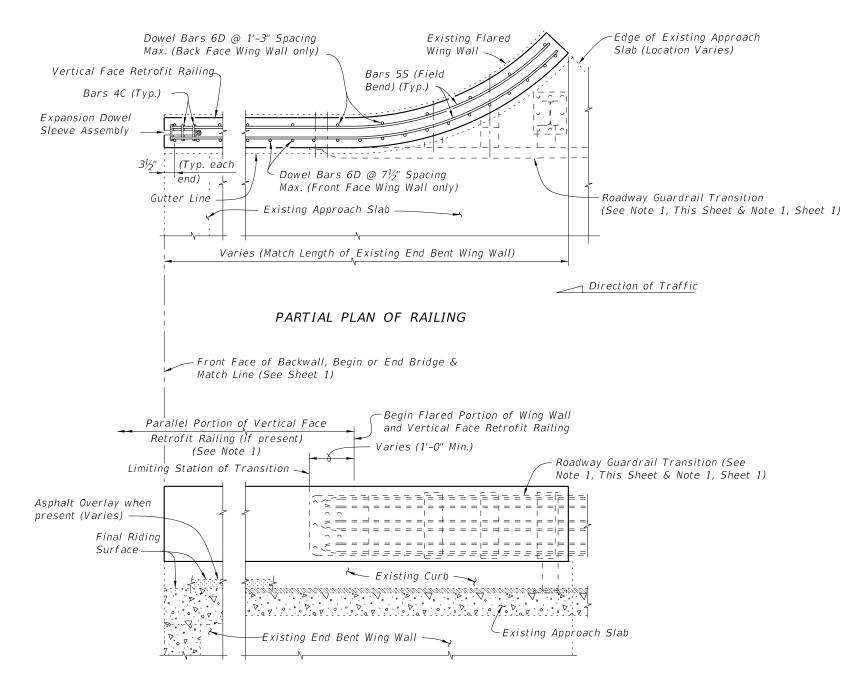


FY 2022-23 STANDARD PLANS

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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

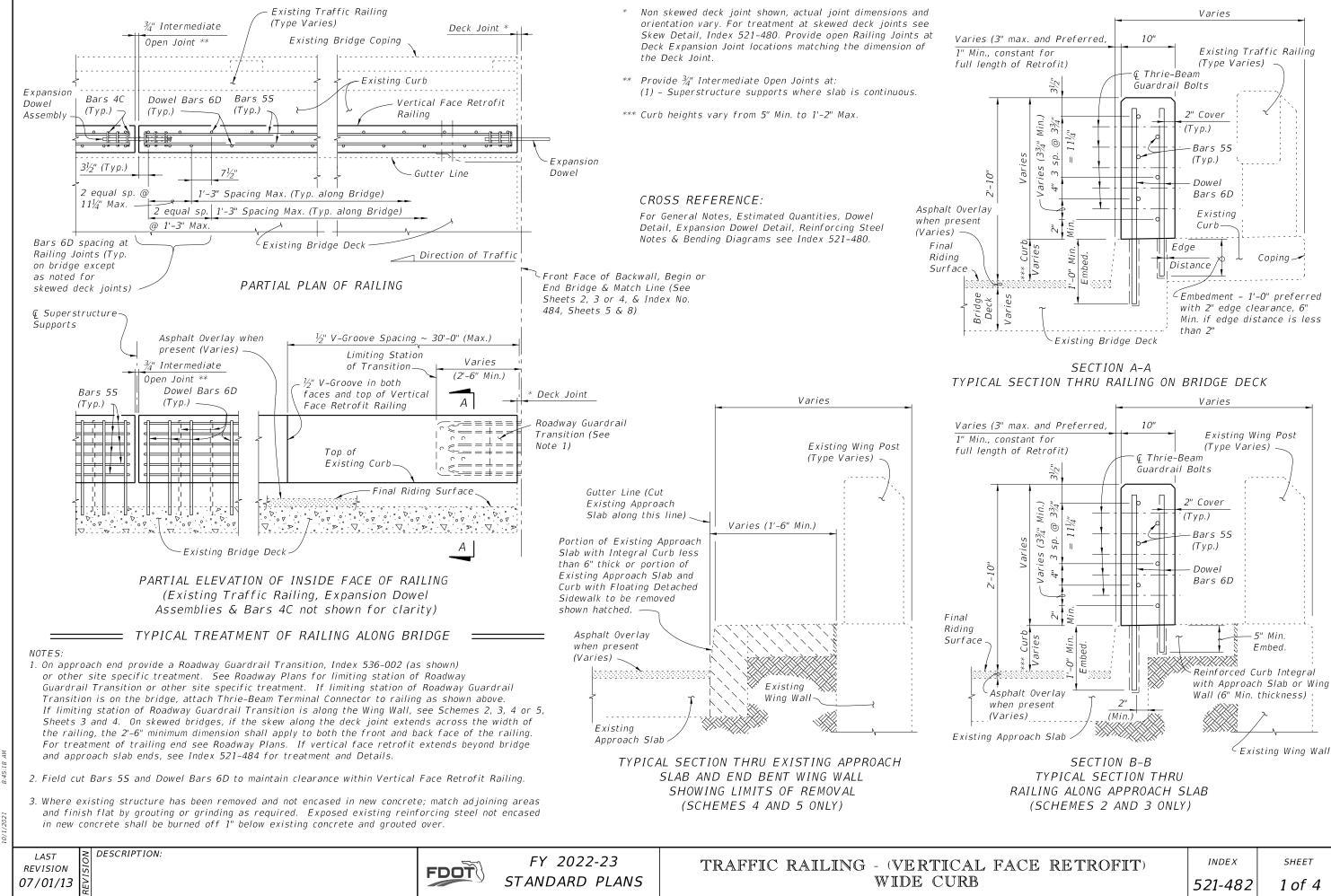
> \_\_\_\_\_ SCHEME 3 \_\_\_\_\_ RAILING END TREATMENT FOR FLARED WING WALLS

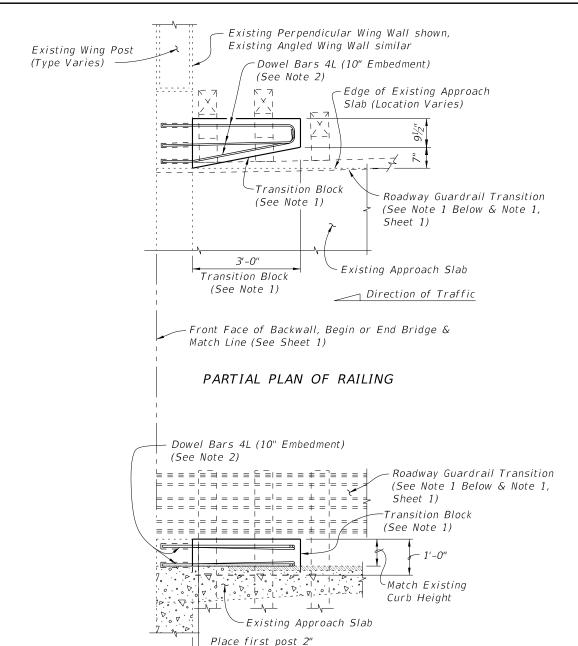
#### SCHEME 3 NOTE:

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see

DESCRIPTION: LAST **REVISION** 07/01/07







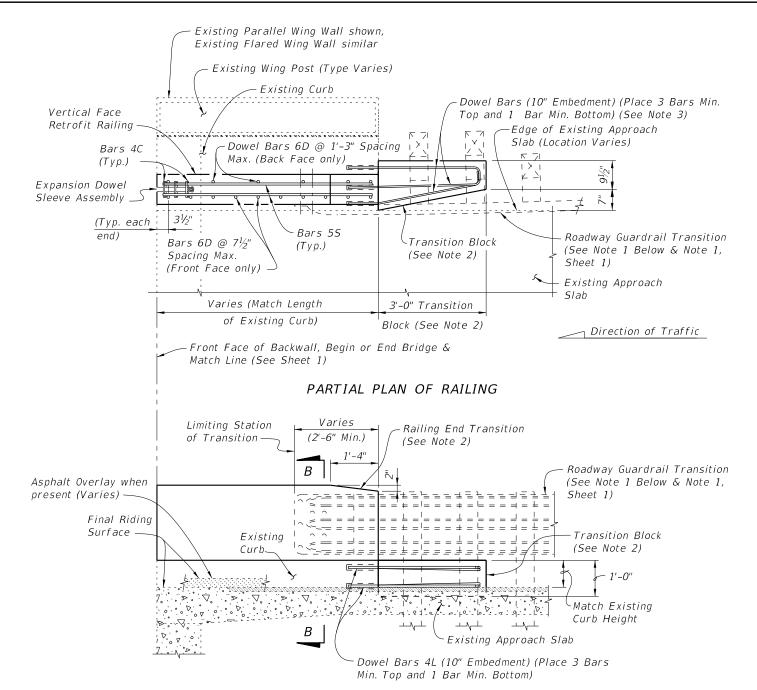
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

clear of Wing Wall

### SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> ====== SCHEME 2 === RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

REVISION 07/01/05

FDOT

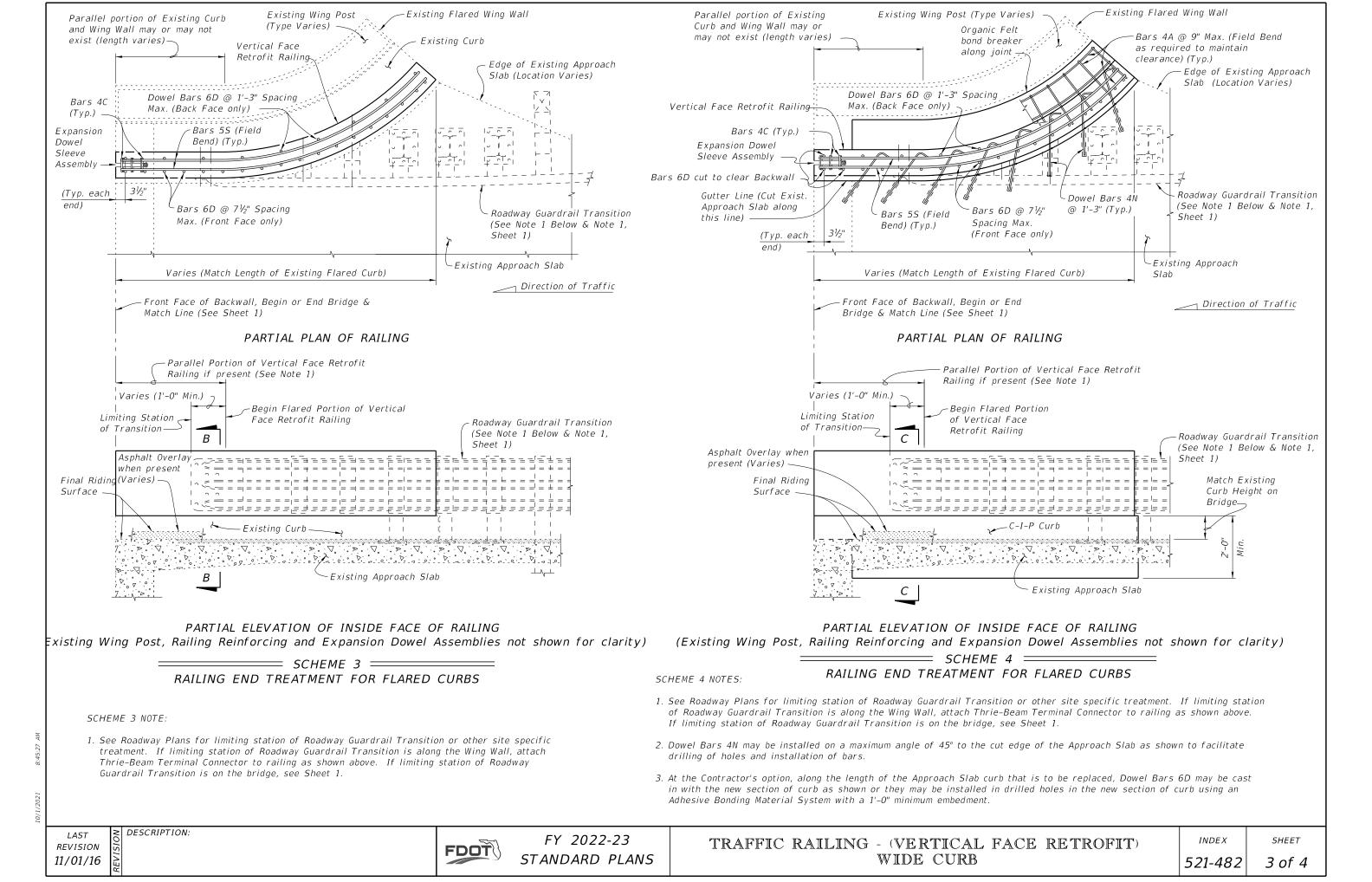
FY 2022-23 STANDARD PLANS

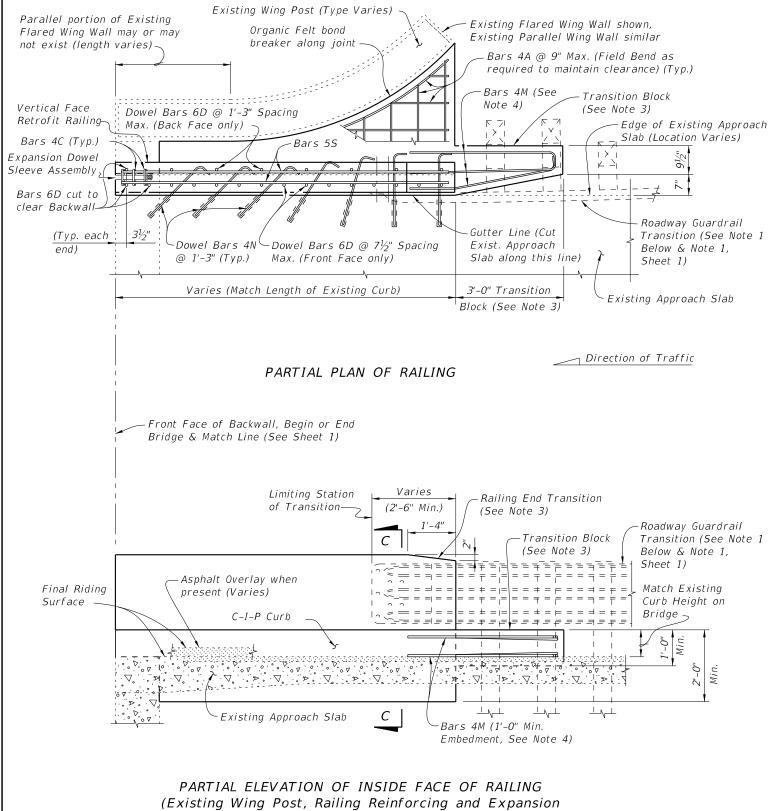
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) WIDE CURB

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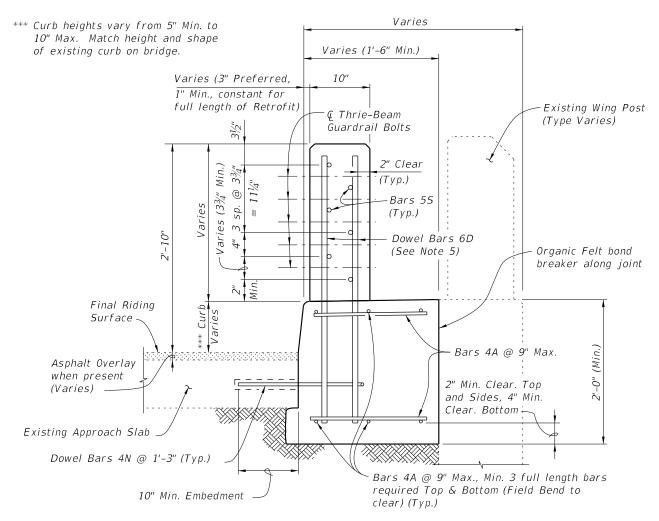
*521-482* 2 of 4





Dowel Assemblies not shown for clarity)

\_\_\_\_\_\_ SCHEME 5 \_\_\_\_\_ RAILING END TREATMENT FOR PARALLEL CURBS

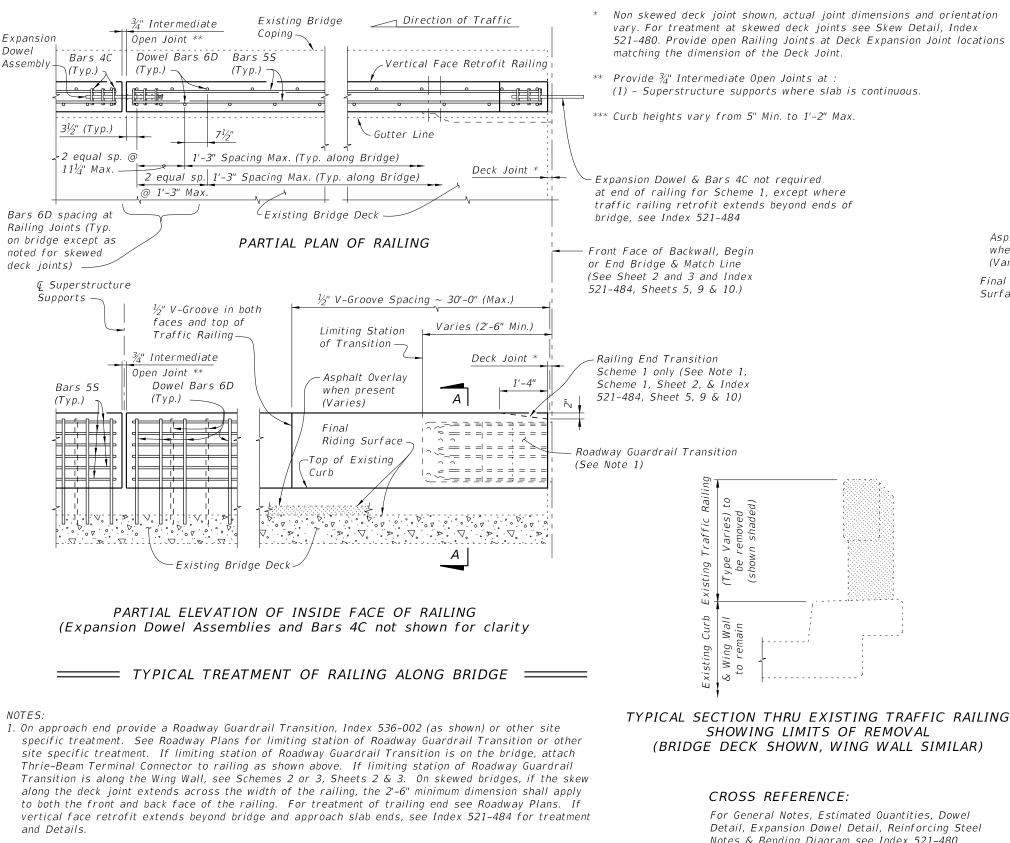


SECTION C-C TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

#### SCHEME 5 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.

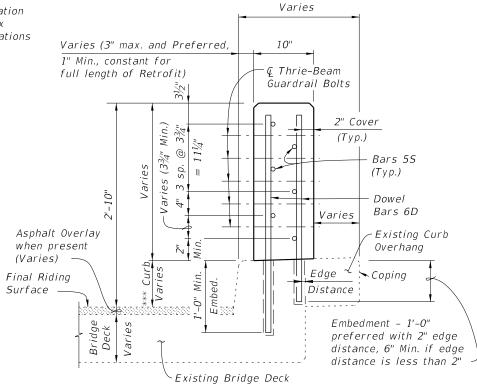
DESCRIPTION:



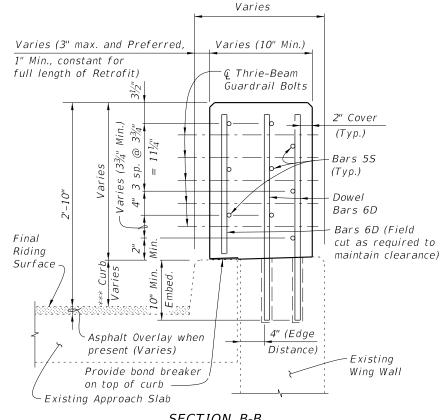
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.
- 3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off 1" below existing concrete and grouted over.

521-480. Provide open Railing Joints at Deck Expansion Joint locations

Notes & Bending Diagram see Index 521-480.



## SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK



SECTION B-B TYPICAL SECTION THRU RAILING ON WING WALL

REVISION 07/01/13

DESCRIPTION:

**FDOT** 

FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) INTERMEDIATE CURB

INDEX

SHEET 1 of 3

*521-483* 

## PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

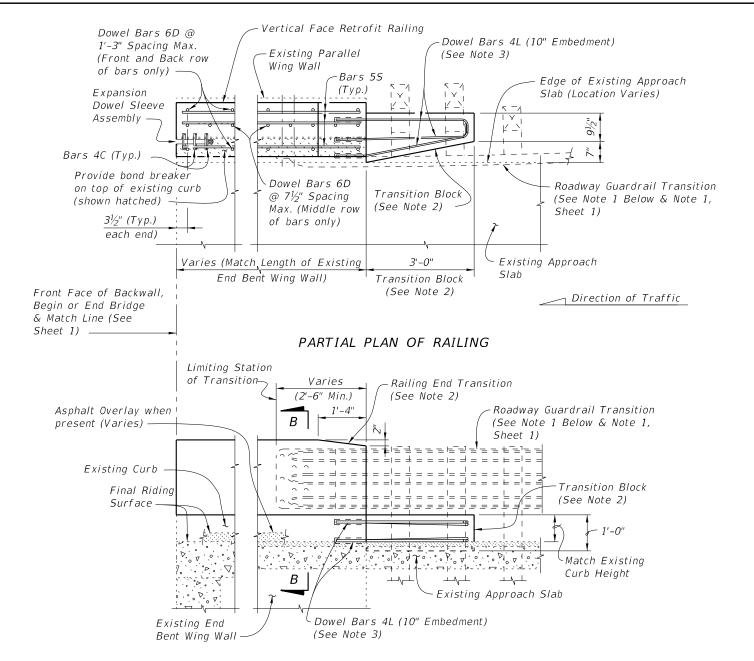
# RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

clear of Wing Wall

#### SCHEME 1 NOTES:

DESCRIPTION:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

## RAILING END TREATMENT FOR PARALLEL WING WALLS

#### SCHEME 2 NOTES:

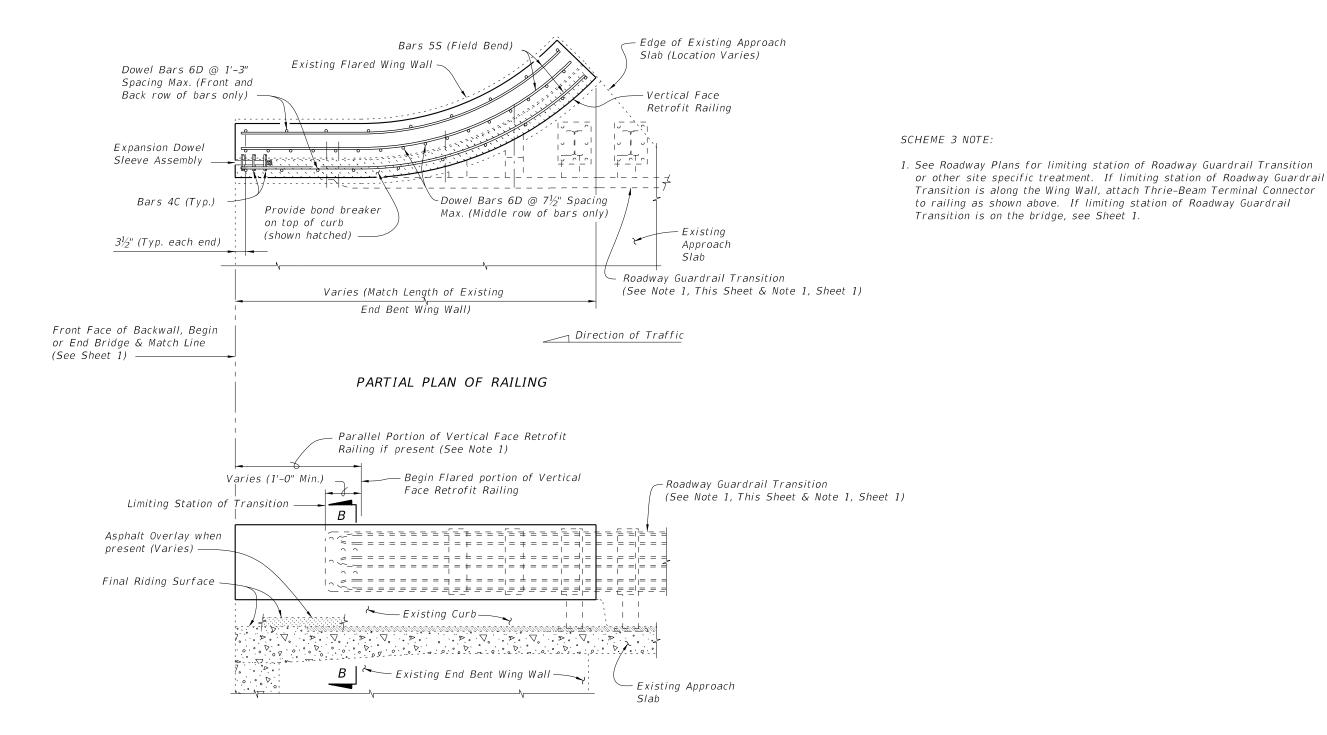
- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

8:45:39

10/1/2021

LAST REVISION 07/01/07

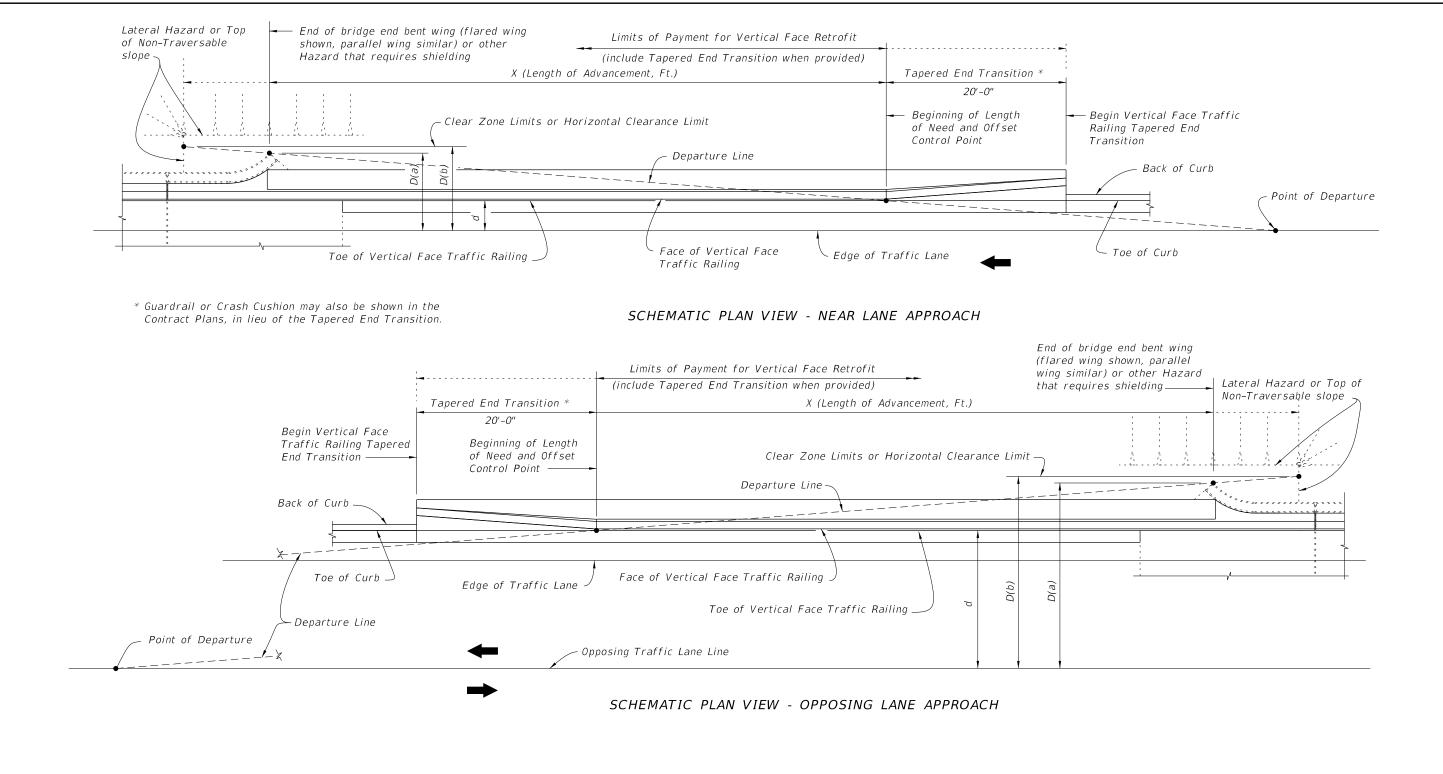




PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> RAILING END TREATMENT FOR FLARED WING WALLS

DESCRIPTION: **REVISION** 07/01/07



#### CROSS REFERENCES:

For General Notes, Dowel Details, Expansion Dowel Details, Reinforcing Steel Notes and Reinforcing Steel Bending Diagram see Index 521-480.

DESCRIPTION: **REVISION** 07/01/09

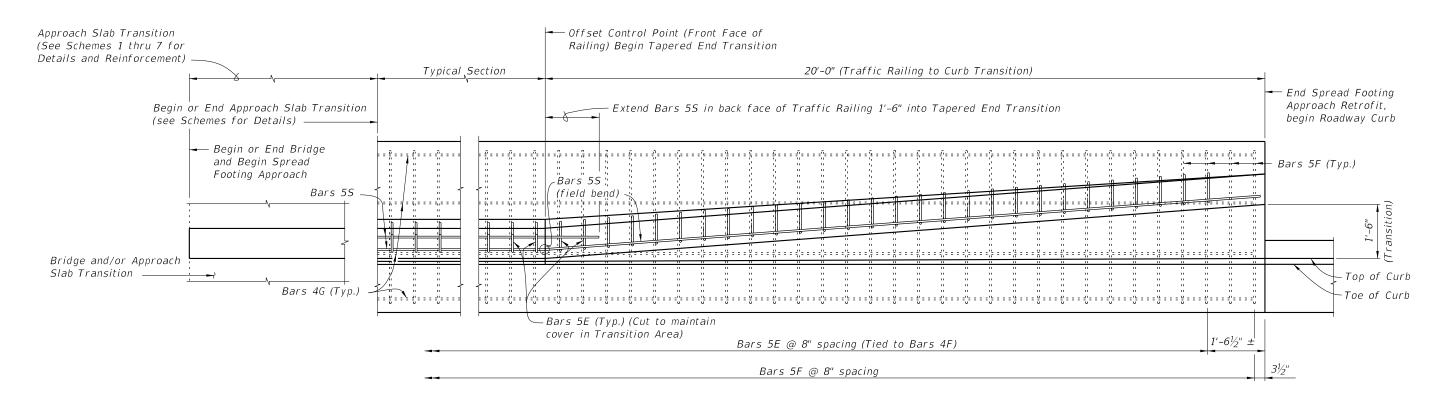
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FY 2022-23 STANDARD PLANS

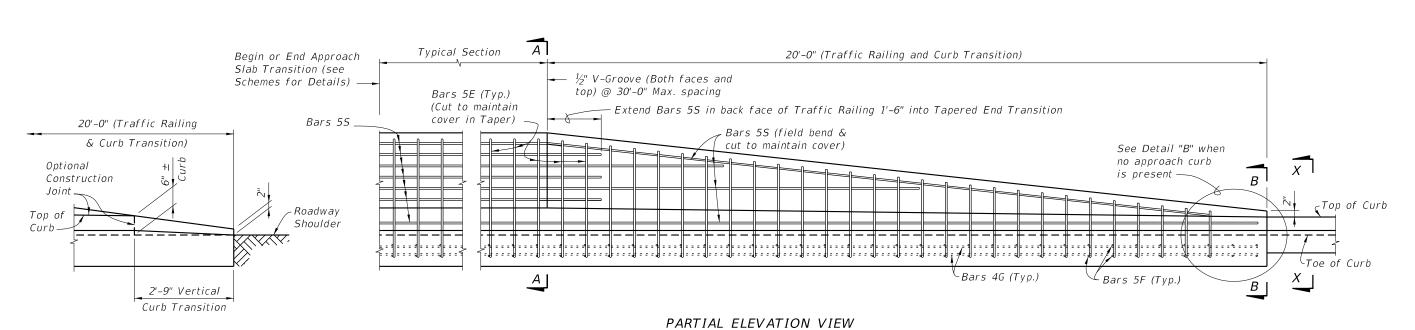
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

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### PARTIAL PLAN VIEW



DETAIL "B" TRANSITION TO NON-CURB APPROACH (Reinforcing Not Shown For Clarity)

*TAPERED END TRANSITION* =

CROSS REFERENCES: For Section A-A, B-B and X-X see Sheet 4.

**REVISION** 07/01/09

DESCRIPTION:

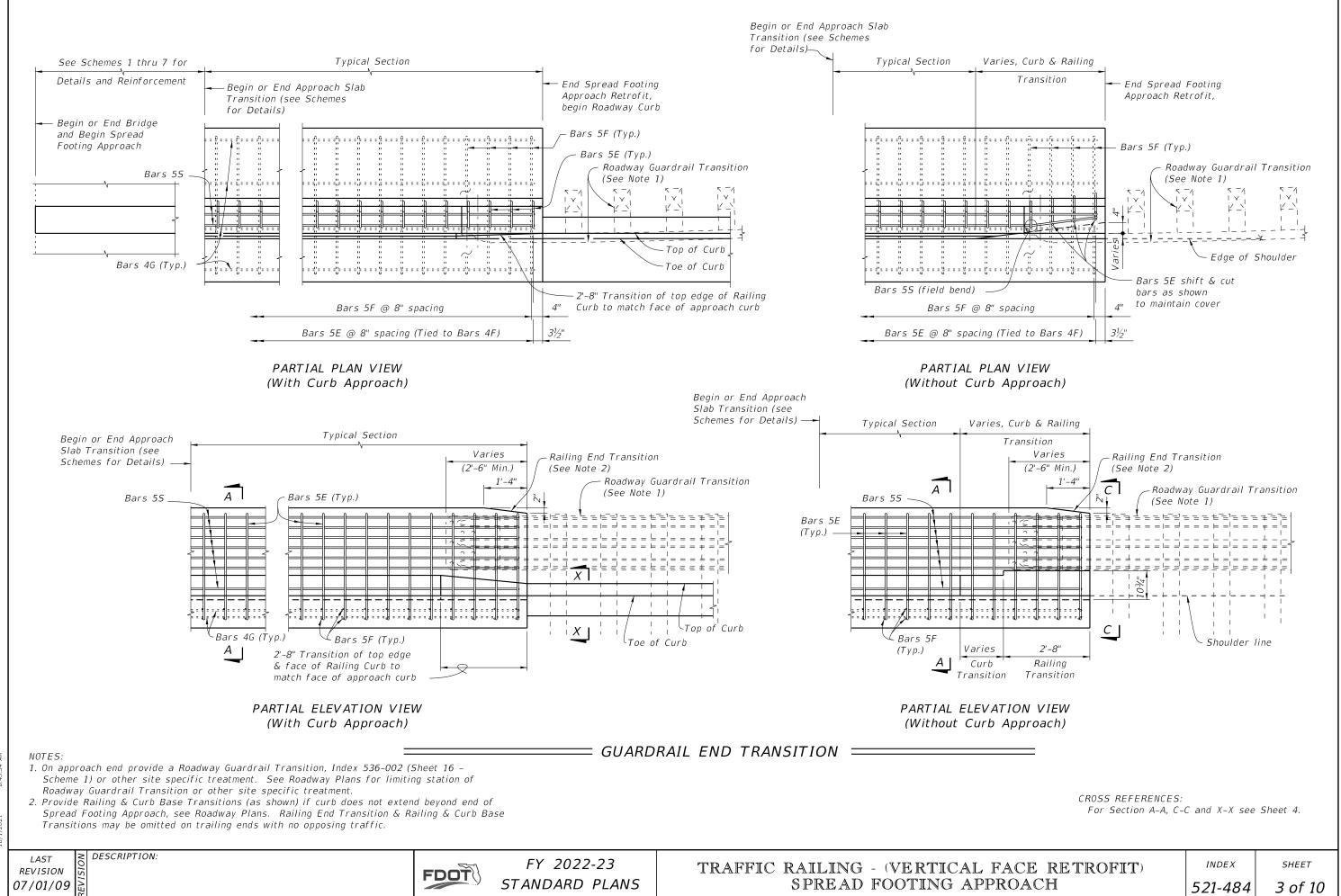
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TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

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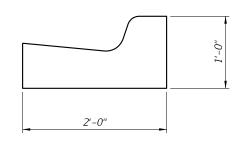
SHEET



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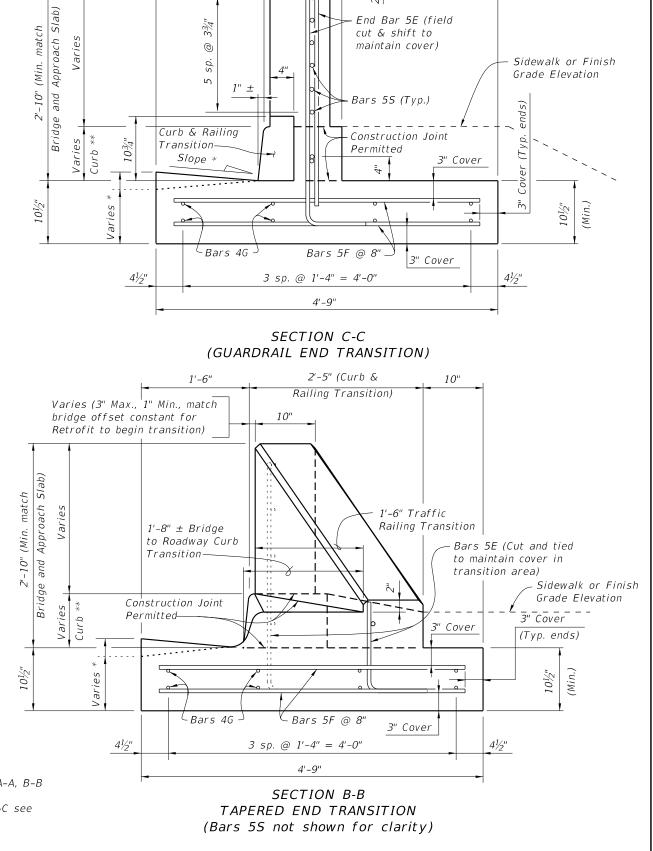
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES				
ITEM	UNIT	QUANTITY		
11 EM	OWIT	9" Curb		
Concrete - Typical Section	CY/Ft.	0.25		
Reinforcing Steel – Typical Section	Lb./Ft.	38		
Concrete - 20'-0" Tapered End Transition plus Footing	CY	4.57 Total		
Reinforcing Steel – 20'-0" Tapered End Transition plus Footing	Lb.	776 Total		

NOTE: Quantities are based on a 9" curb, no curb cross slope.



SECTION X-X (TYPICAL CURB, TYPE VARIES, TYPE F SHOWN) (See Index 520-001 and Plans for Details)

- \* Match Cross Slope of high side and low side at begin or end bridge or approach
- \*\* Match curb height of adjacent bridge and approach slab. Adjust height in Transition area to match adjoining Roadway curb.



1'-1"

10"

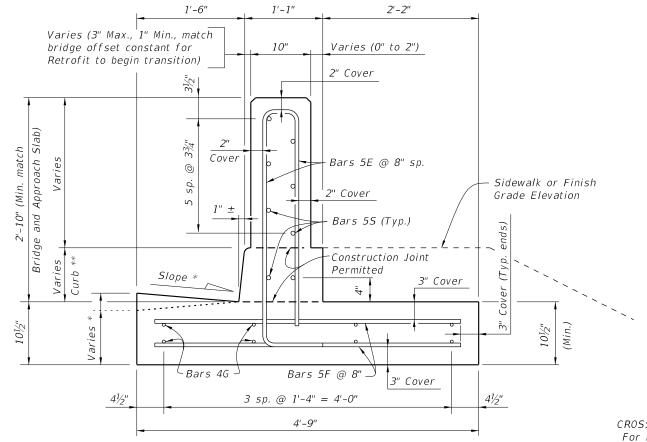
1'-6"

Varies (3" Max., 1" Min., match

bridge offset constant for Retrofit to begin Curb & Railing Transition) 2'-2"

Varies (0" to 2")

– End Bar 5E (field cut & shift to

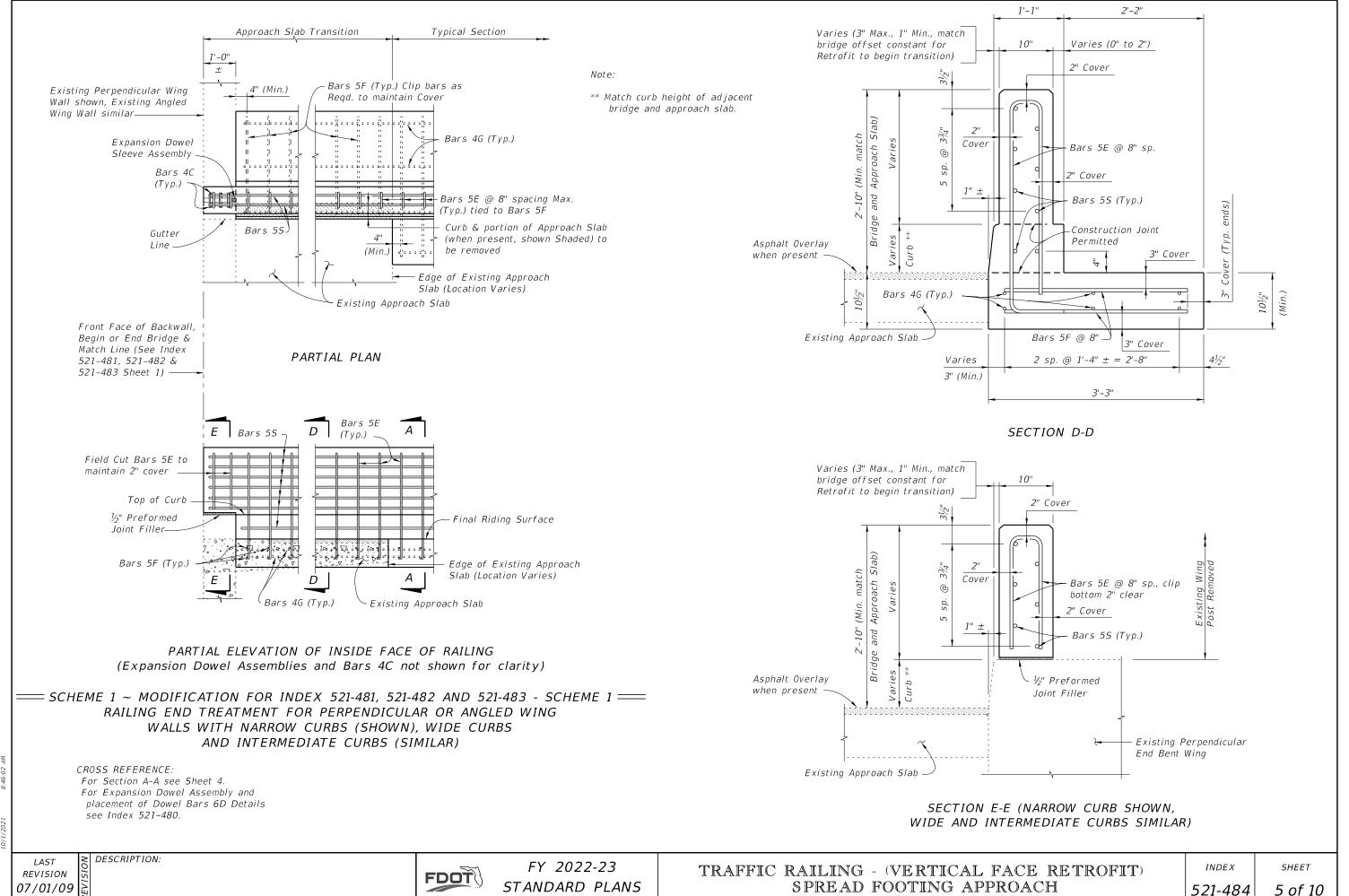


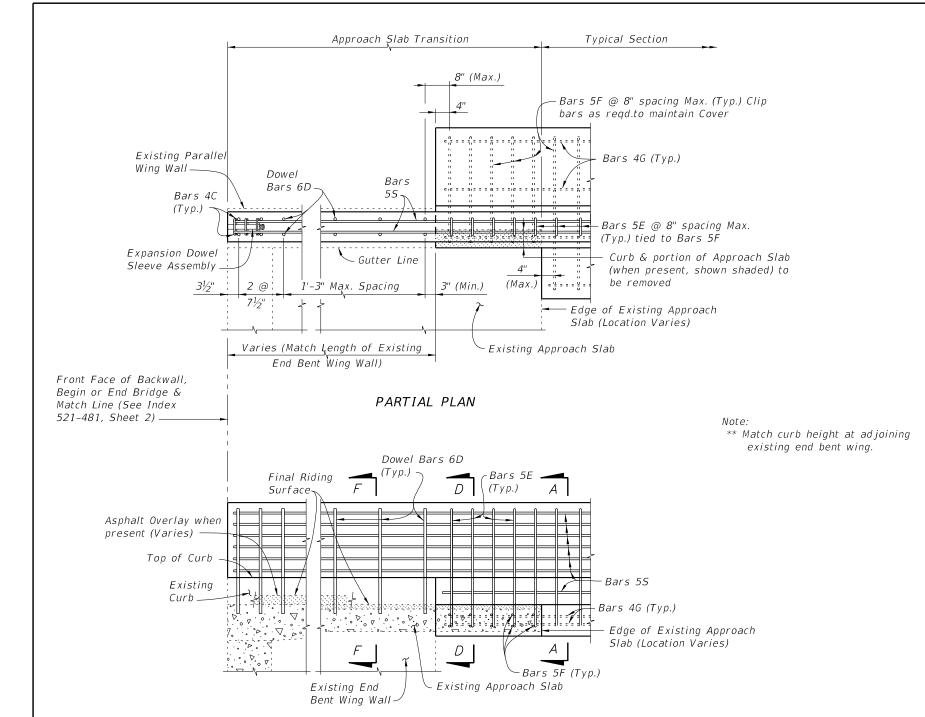
SECTION A-A TYPICAL SECTION (9" Curb shown, 6" Curb similar) CROSS REFERENCES: For location of Sections A-A, B-B and X-X see Sheet 2. For location of Section C-C see Sheet 3.

DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

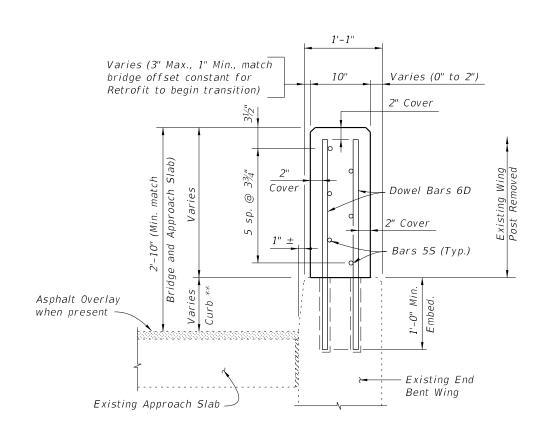




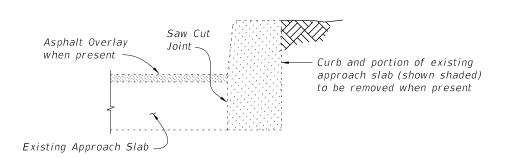
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

#### NOTES:

1. Remove existing concrete along saw cut joints. Existing reinforcing steel may be cut at joint or extended into new concrete. Exposed existing reinforcing not encased in new concrete shall be removed 1" below existing concrete surface and grouted over.



## SECTION F-F



SECTION THRU EXISTING CURB AND APPROACH SLAB TO BE REMOVED (Free Standing Curb Similar)

#### CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

LAST REVISION 07/01/09

DESCRIPTION:

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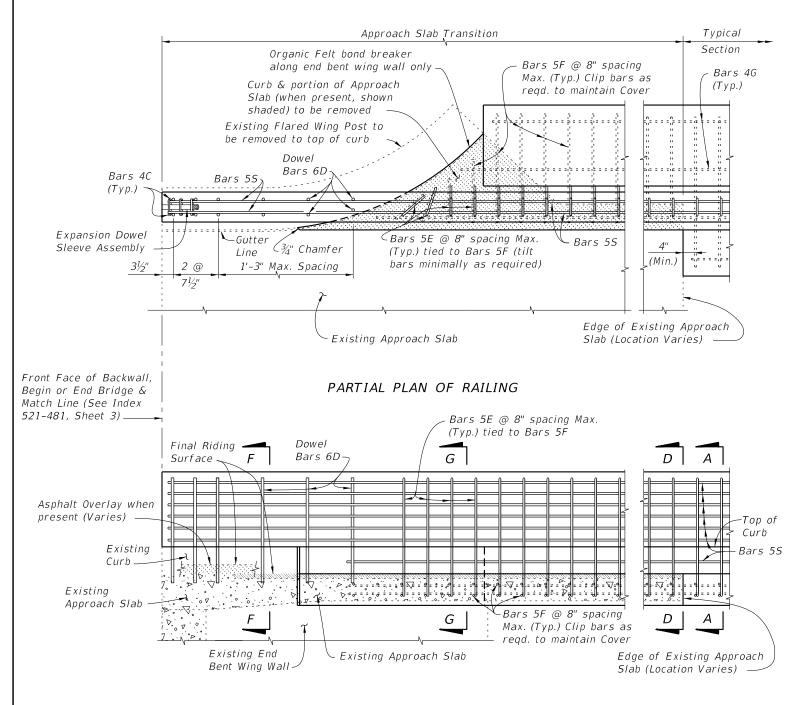
FY 2022-23 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
SPREAD FOOTING APPROACH

INDEX 521-484

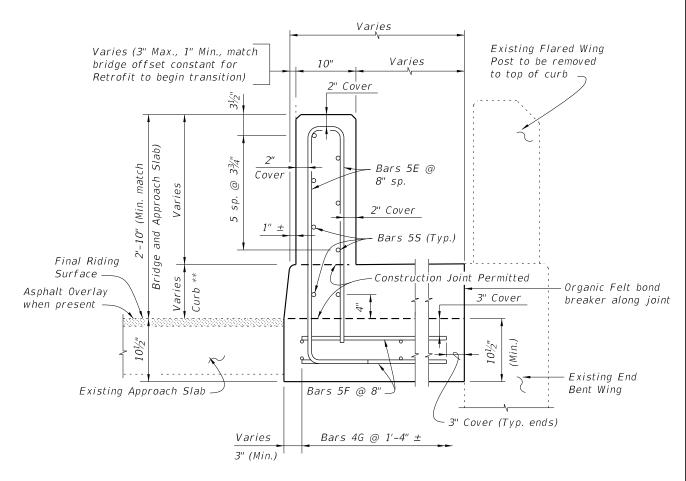
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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

SCHEME 3 ~ MODIFICATION FOR INDEX 521-481 SCHEME 3 ====== RAILING END TREATMENT FOR FLARED WING WALLS WITH NARROW CURBS



SECTION G-G

Note:

\*\* Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4.

For Section D-D see Sheet 5. For Section F-F see Sheet 6.

For Expansion Dowel Assemblies Details and placement of Dowel Bars 6D see Index 521-480.

REVISION 11/01/16

DESCRIPTION:

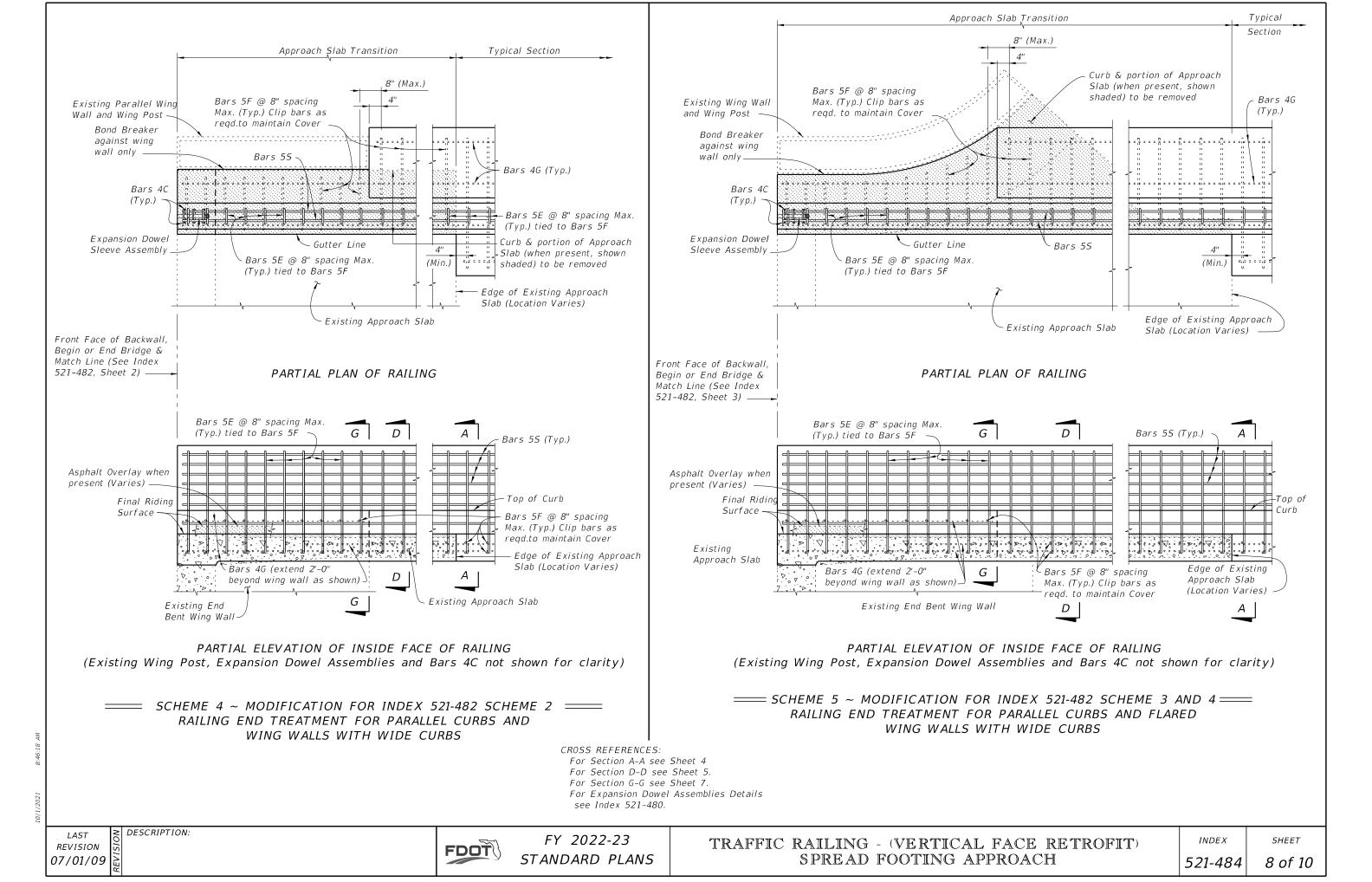
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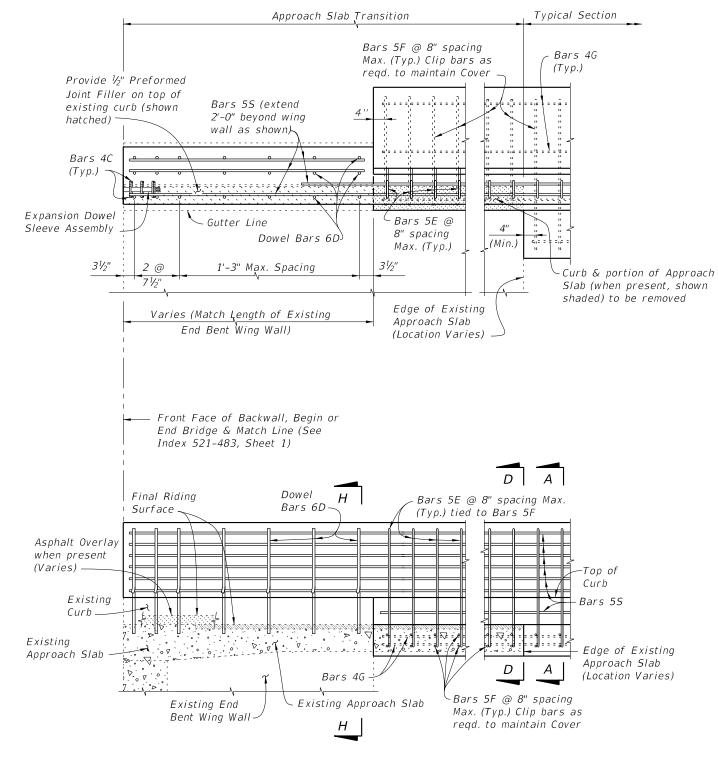
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

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SHEET

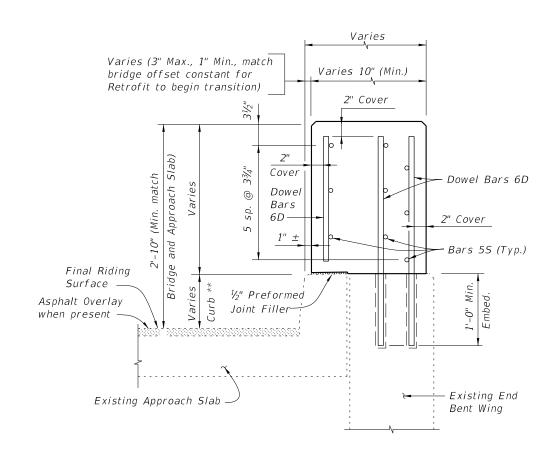
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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 6 ~ MODIFICATION FOR INDEX 521-483 SCHEME 2 ===== RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH INTERMEDIATE CURBS



SECTION H-H

Note:

\*\* Match curb height at adjoining existing end bent wing.

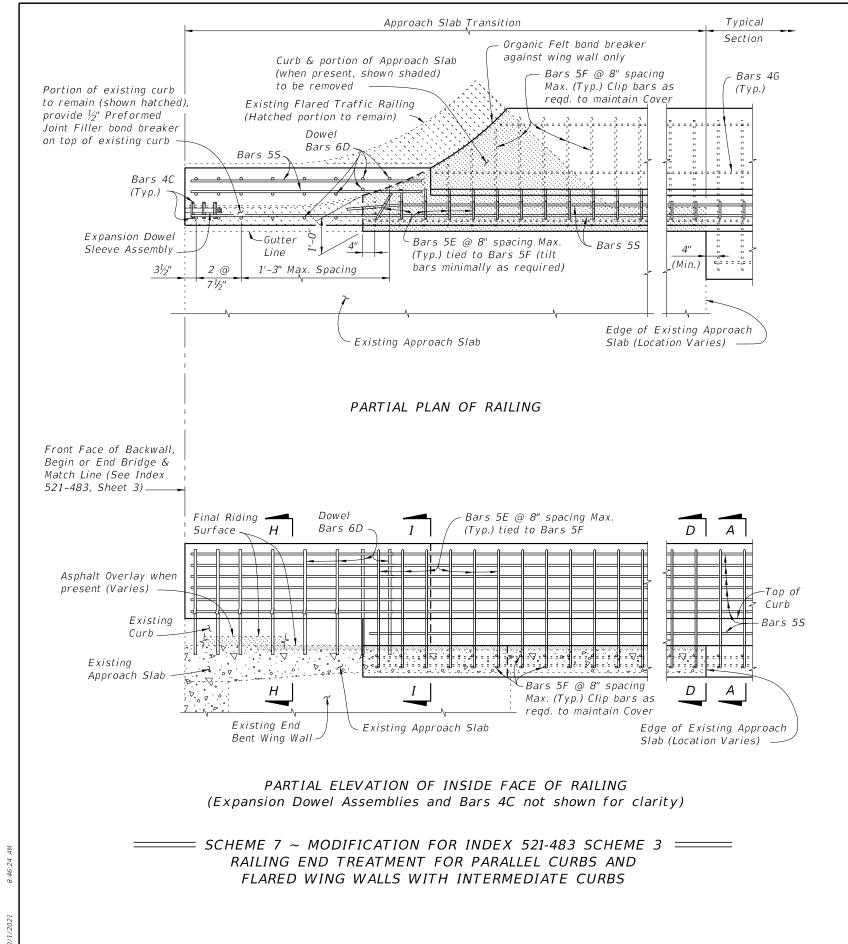
CROSS REFERENCES:

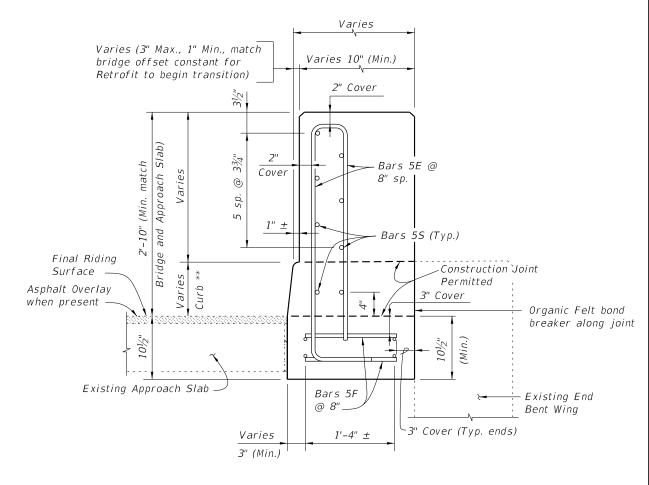
For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

**REVISION** 07/01/09

DESCRIPTION:

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#### SECTION I-I

\*\* Match curb height at adjoining existing end bent wing.

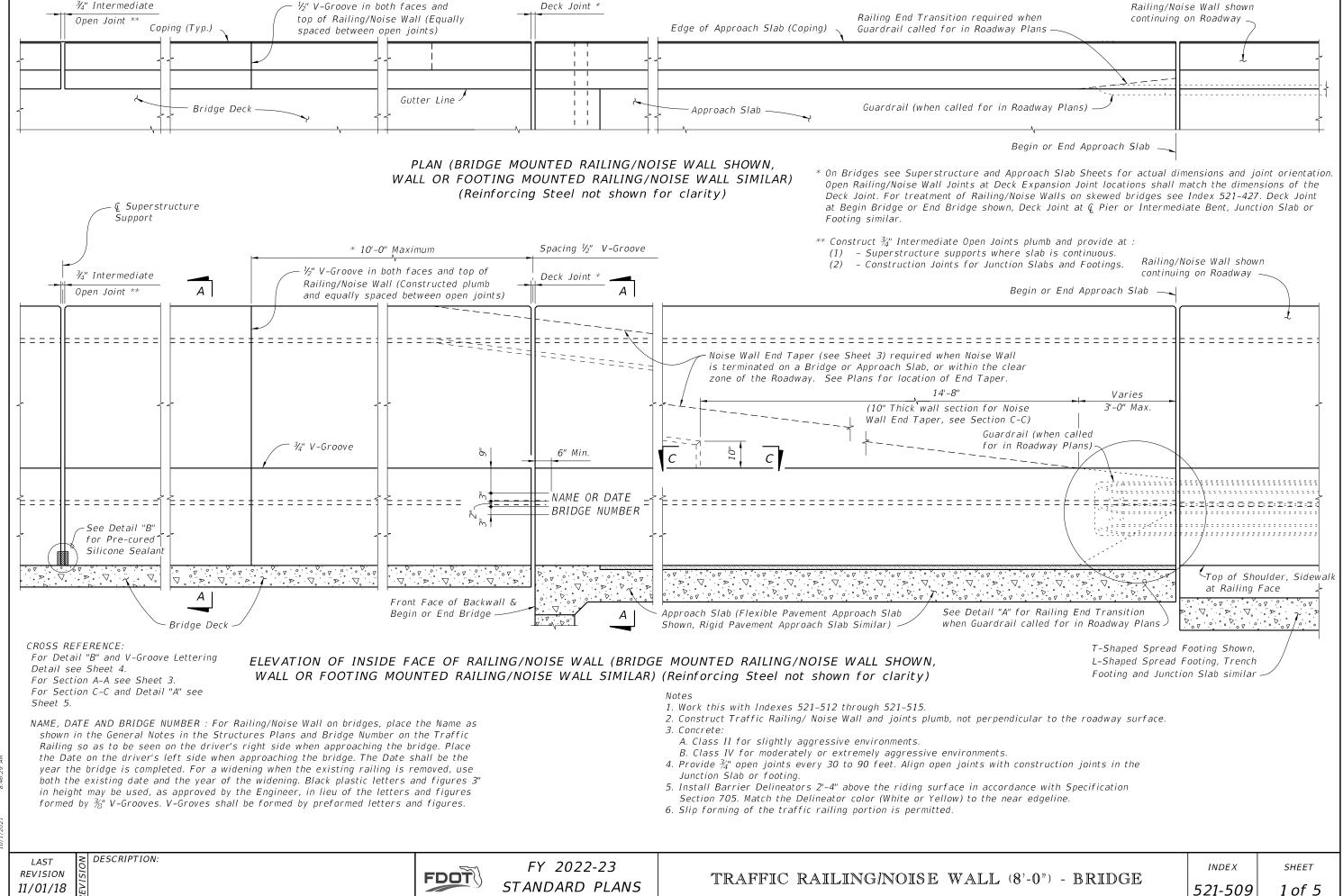
CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section H-H see Sheet 9. For Expansion Dowel Assemblies and placement of Dowel Bars 6D Details see Index 521-480.

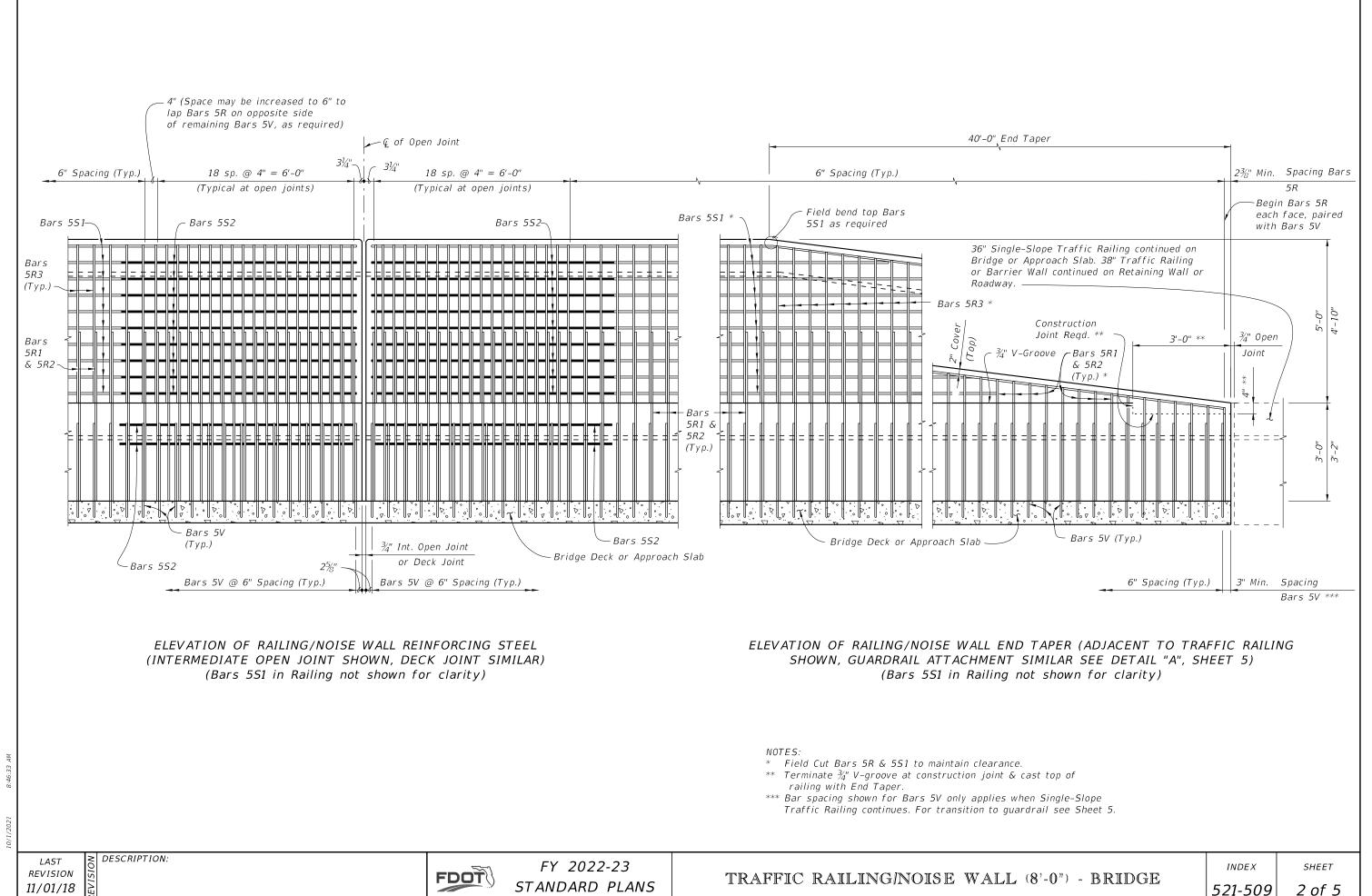
REVISION 11/01/16

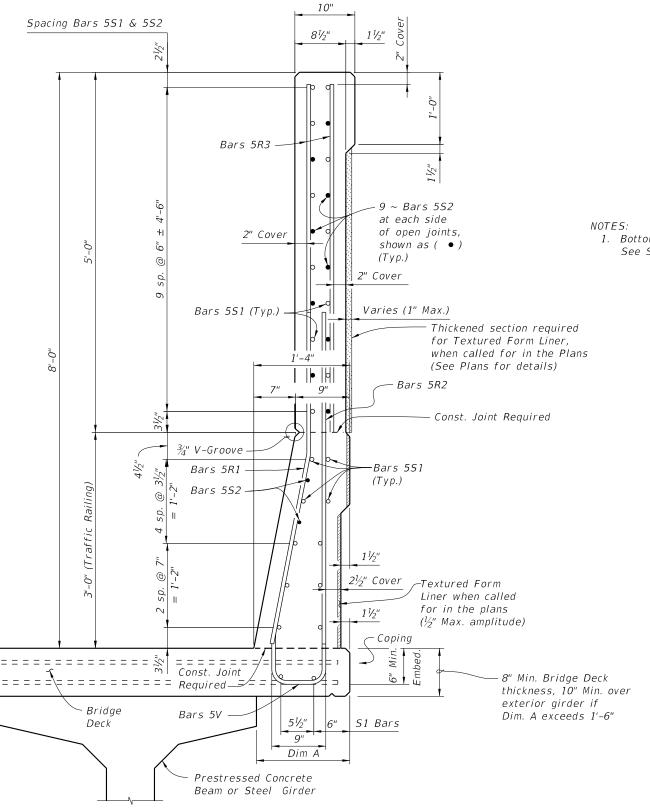
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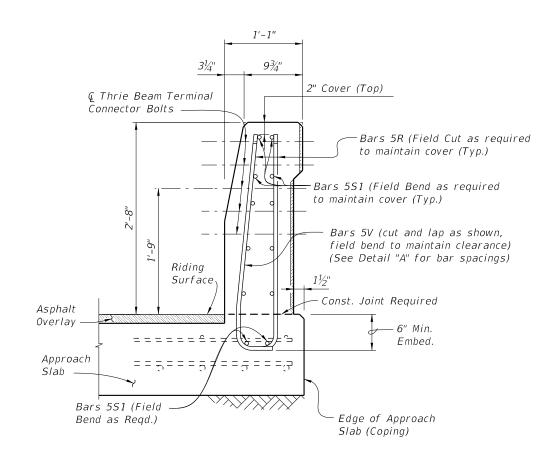


SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING/NOISE WALL (Section Thru Bridge Deck Shown, Section Thru Approach Slab Similar)

CROSS REFERENCE:

For locations of Section A-A see Sheet 1. For location of View B-B, see Sheet 5.

Bottom Bars 5S1 shown are part of the Traffic Railing/Noise Wall reinforcing.
 See Superstructure Sheets in the Plans for additional Bridge Deck Reinforcing.



VIEW B-B
END VIEW OF RAILING END TRANSITION FOR
GUARDRAIL ATTACHMENT AT END OF APPROACH SLAB
(Flexible Pavement Approach Slab Shown, Rigid Pavement Approach Slab Similar)

LAST REVISION 11/01/18

DESCRIPTION:

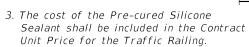
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FY 2022-23 STANDARD PLANS TRAFFIC RAILING/NOISE WALL (8'-0") - BRIDGE

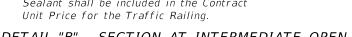
SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

#### INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

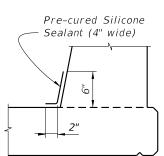


DESCRIPTION:



ESTIMATED TRAFFIC RAILING/NOISE WALL QUANTITIES				
ITEM UNIT QUANTIT				
Concrete (Railing)	CY/LF	0.107		
Concrete (Noise Wall)	CY/LF	0.136		
Reinforcing Steel (Typical)	LB/LF	69.36		
Additional Reinf. @ Open Joint	LB	226.85		

(The above quantities are based on the bridge mounted typical section, 2% deck cross slope and railing on low side of deck.)



Pre-cured Silicone — Sealant (4" wide)
2"

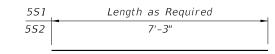
## DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

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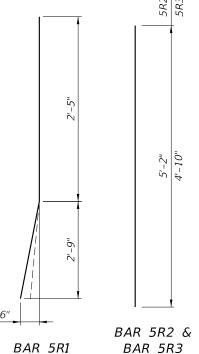
## REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL					
MARK	SIZE	LENGTH			
R1	5	5'-2"			
R2	5	5'-2 <sup>1</sup> / <sub>2</sub> "			
R3	5	4'-10"			
<i>51</i>	5	As Reqd.			
52	5	7'-3"			
V	5	6'-6 <sup>1</sup> / <sub>5</sub> "			

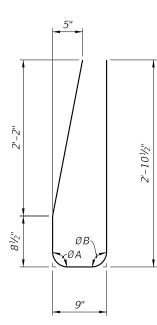
BRIDGE		LOW G	UTTER	HIGH C	GUTTER
CR	OSS-SLOPE	ØA	ØB	ØA	ØB
E :D	0% to 2%	90°	90°	90°	90°
BRIDGE MOUNTED	2% to 6%	93°	87°	87°	93°
B) M0	6% to 10%	96°	84°	84°	96°



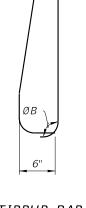
BARS 5S1 & 5S2



(Field Cut and Bend for Railing End Transition)



STIRRUP BAR 5V



END STIRRUP BAR 5V To Be Field Cut (Railing End Transition)

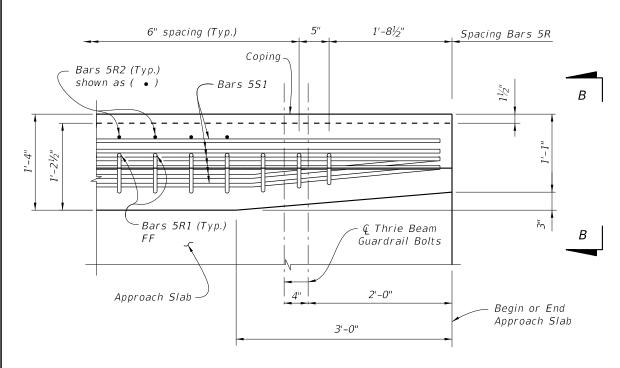
#### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.
- 4. Bars 5S1 may be continuous or spliced at the construction joints. Lap splices for Bars 5R2 and 5S1 shall be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

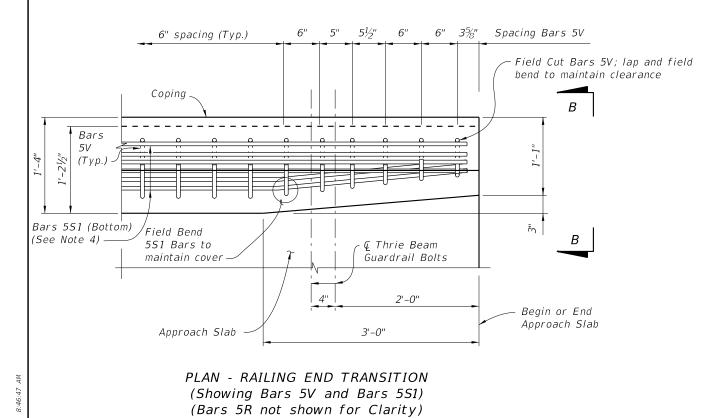
CROSS REFERENCE: For locations of Detail "B", see Sheet 1.

**REVISION** 

11/01/18



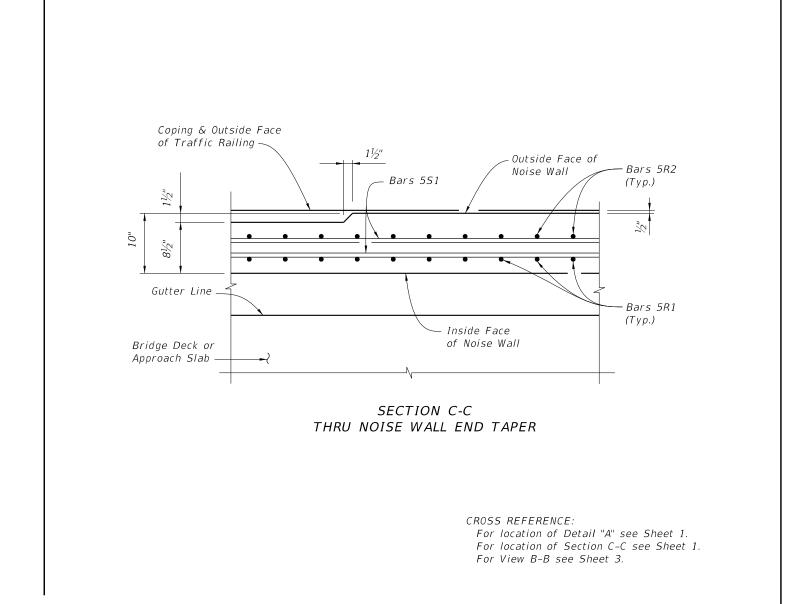
PLAN - RAILING END TRANSITION (Showing Bars 5R, and Bars 5S1) (Bars 5V & Noise Wall Reinforcement not shown for Clarity)



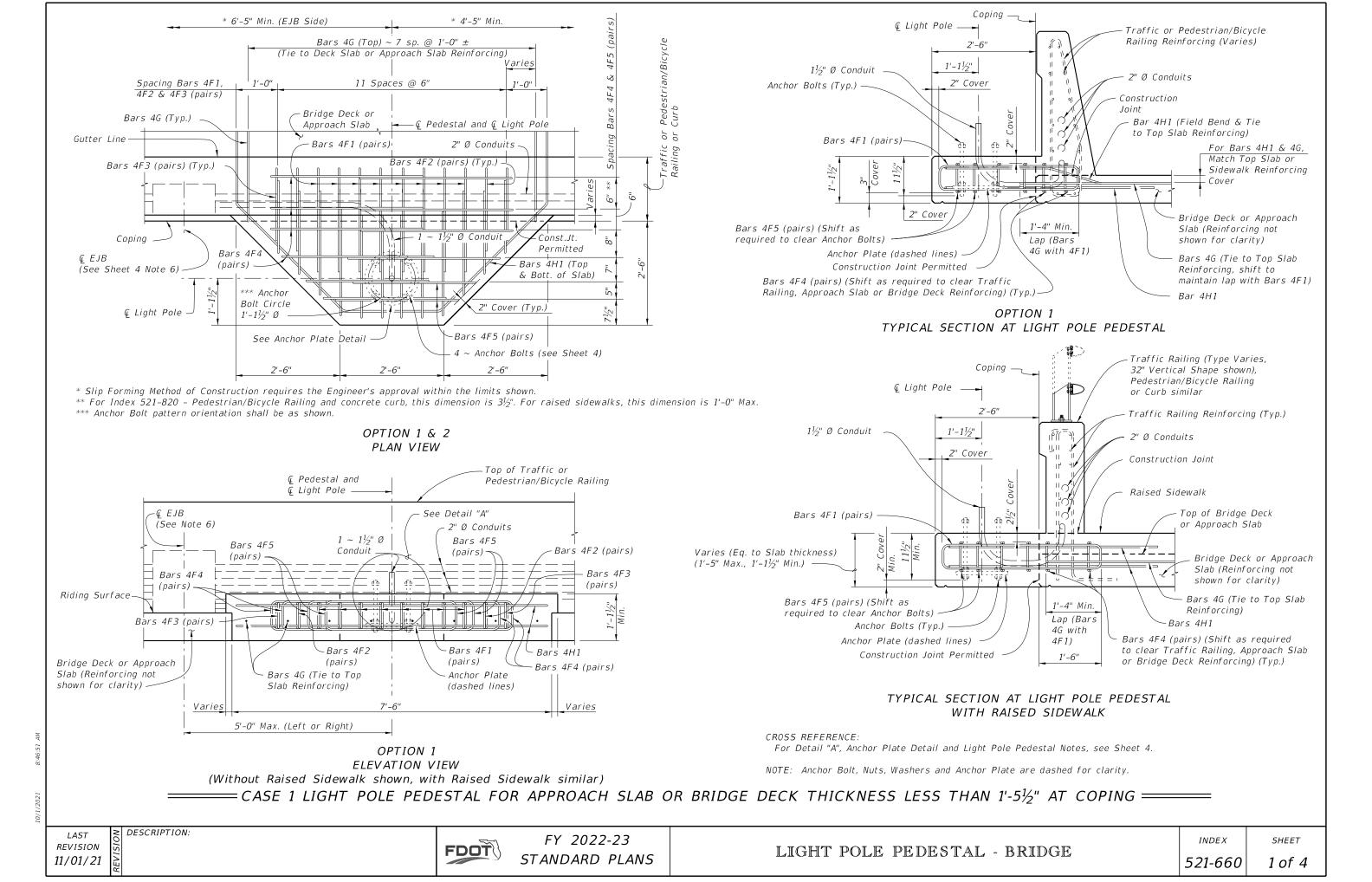
DETAIL "A"

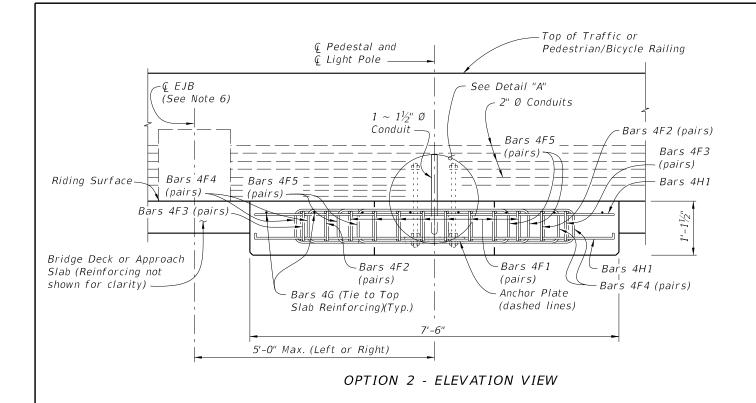
#### DETAIL "A" NOTES:

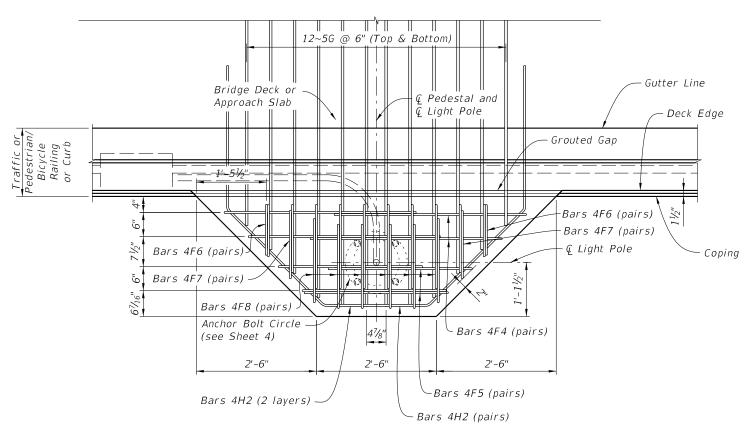
- 1. Begin placing Railing Bars 5V at the railing end and proceed toward the guardrail (thrie beam) terminal connector to ensure placement of guardrail bolt holes. Pair Bars 5R with Bars 5V as shown. Clearance of Bars 5R & 5V to guardrail bolt holes shall be checked to prevent cutting of bars if holes are to be drilled. Shift bars locally where conflicts occur.
- 2. For Guardrail connection details see Index 536-001.
- 3. Omit Railing End Transition if a 36" Single-Slope Traffic Railing is used beyond the End Taper. See the Plan Sheets.
- 4. Field cut Bars 5R2 to maintain cover. Field cut Bars 5V and lap as necessary to maintain cover; field cut & bend Bars 5R1 front leg (more plumb) to maintain cover and tie to S1 Bars.



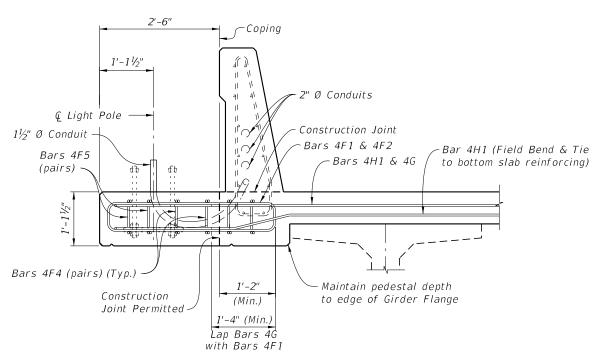
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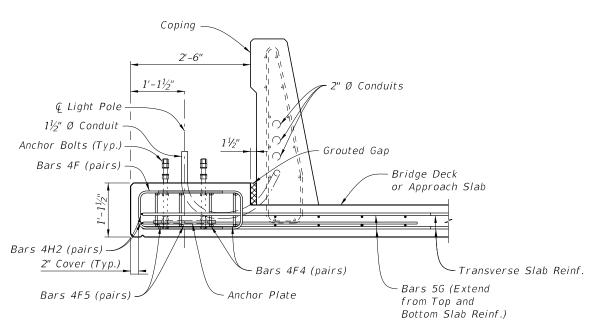




OPTION 3 - PLAN VIEW WITH GAP BETWEEN BARRIER AND PEDESTAL TO ALLOW SLIP FORMING



OPTION 2 - TYPICAL SECTION AT LIGHT POLE PEDESTAL (Approach Slab Similar)



OPTION 3 - TYPICAL SECTION AT LIGHT POLE PEDESTAL WITH GAP BETWEEN BARRIER AND PEDESTAL TO ALLOW SLIP FORMING

CROSS REFERENCE:

For Detail "A", Anchor Plate Detail and Light Pole Pedestal Notes, see Sheet 4.

NOTE: Anchor Bolt, Nuts, Washers and Anchor Plate are dashed for clarity.

= CASE 1 LIGHT POLE PEDESTAL FOR APPROACH SLAB OR BRIDGE DECK LESS THAN 1'-5 $^1\!\!/_2$ " AT COPING  $=\!=\!=\!=\!=\!=$ 

**REVISION** 11/01/21

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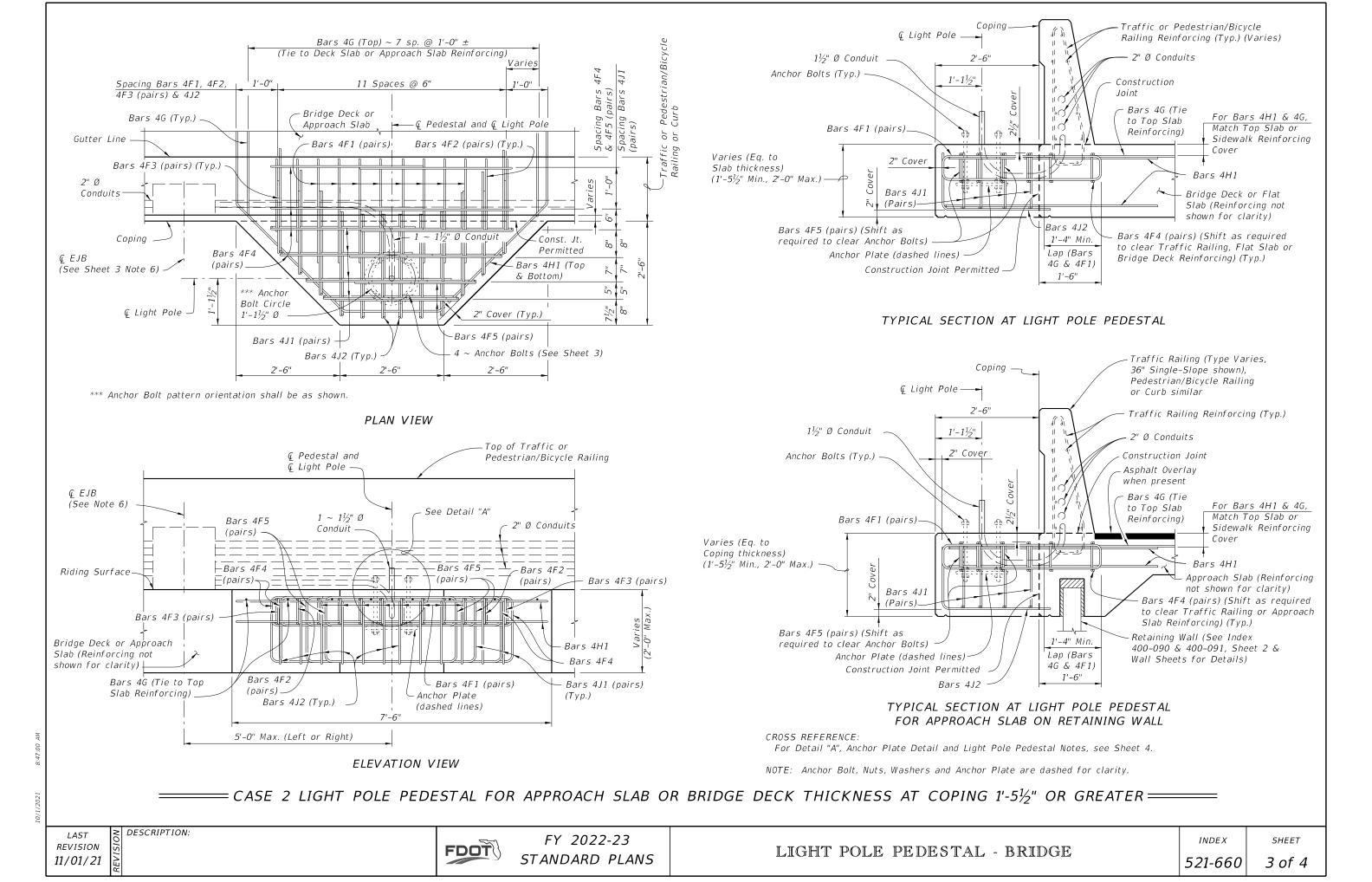
LIGHT POLE PEDESTAL - BRIDGE

INDEX

SHEET

DESCRIPTION:

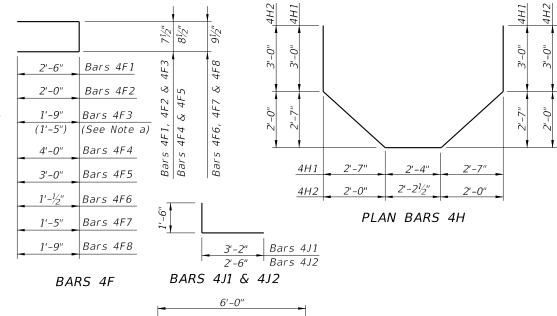
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## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

#### REINFORCING STEEL NOTES:

- a. When Pedestal is attached to Pedestrian/Bicycle Railing Index 521-820 or an 8" wide concrete curb and the Bridge Deck or Approach Slab thickness is less than  $1'-1\frac{1}{2}''$ , Bars 4F3 shall have leg length and bar length shown in parentheses.
- b. The number of bars shown in parentheses is for Bars 4F4 when Pedestal is attached to Pedestrian/Bicycle Railing - Index 521-820 or an 8" wide concrete curb, and the Bridge Deck or Approach Slab thickness is less than  $1'-1\frac{1}{2}''$ .
- c. Lap Splices for Bars 4F1, 4F2 & 4F3 shall be a minimum of 1'-4". Lap Splices for Bars 4F4 & 4F5 shall be minimum of 1'-8".
- d. Bars 4J1 and 4J2 are not required when Pedestal thickness is less than  $1'-5\frac{1}{3}$ ". Field trim height of bars to maintain cover when Pedestal thickness is less than 2'-0". Field trim length of Bars 4J2 on Retaining Wall Coping to maintain cover.
- e. All bar dimensions in the bending diagrams are out to out.



BILL OF REINFORCING STEEL				
MARK	SIZE	NO. REQD.	LENGTH	NOTES
F 1	4	16	5'-8"	С
F2	4	4	4'-8"	С
F3	4	4	4'-2'' (3'-6")	а, с
F 4	4	8 (6) [4 for Option 3]	8'-9"	b, c
F5	4	4	6'-9"	С
F6	4	4	2'-11"	-
F7	4	4	3'-8"	-
F8	4	12	4'-4''	-
G	4 [5 for Option 3]	8 [24 for Option 3]	6'-0"	-
H1	4	2	15'-8"	-
H2	4	2	13'-10"	-
J 1	4	8	4'-8"	d
J2	4	12	4'-0''	d

( ) See Reinforcing Steel Note a & b.

## LIGHT POLE PEDESTAL NOTES

1. Concrete and Reinforcing Steel required for the construction of the Pedestal shall meet the same requirements as the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.

BAR 5G

- 2. Light Pole Pedestal may be used with the following: Index 521-422 - Traffic Railing (42" Vertical Shape), Index 521-423 - Traffic Railing (32" Vertical Shape), Index 521-427 - Traffic Railing (36" Single-Slope), Index 521-428 - Traffic Railing (42" Single-Slope), Index 521-820 - Pedestrian/Bicycle Railing, Index 515-021 - Pedestrian/Bicycle Bullet Railing for Traffic Railing or Index 515-509 - Traffic Railing /Noise Wall - Bridge.
- 3. Unless otherwise noted, Traffic Railing (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other Traffic Railings or Pedestrian/Bicycle Railing are similar.

4. ANCHOR BOLTS:

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole configurations shown on Index 715-002.

Anchor Bolt Diameter: See Table 1 Anchor Bolts: ASTM F1554 Grade 55. Nuts: ASTM A563 Grade A, Heavy-Hex.

Washers: ASTM F436 Type 1.

Anchor Plate: ASTM A709 (Grade 36) or ASTM A36.

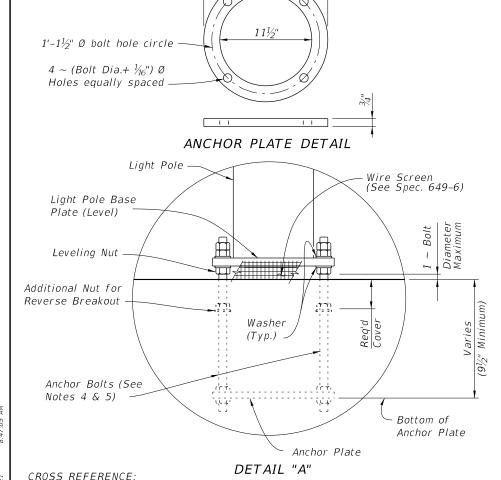
Coating: Galvanize all Nuts, Bolts Washers, in accordance with ASTM F2329. Galvanize plates in accordance with ASTM A123.

The Contractor is responsible for ensuring the anchor bolt configuration is compatible with the light pole base plate. Submit modifications of the anchor bolt design to the Engineer for approval.

- 5. Install Anchor Bolts plumb.
- 6. For Conduit, Embedded Junction Boxes (EJB), Expansion/Deflection Fitting and adjacent Reinforcing Steel Details, see Utility Conduit Detail Sheets and Index 630-010.
- 7. PAYMENT: The cost of Wire Screen, Anchor Bolts, Nuts, Washers and Anchor Plates shall be included in the Bid Price for Light Poles. The cost of all Labor, Concrete and Reinforcing Steel required for the Construction of the Pedestals, and Miscellaneous Hardware required for the completion of the Electrical System, shall be included in the Bid Price for the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.

ESTIMATED LIGHT POLE PEDESTAL QUANTITIES PER LIGHT POLE PEDESTAL			
ITEM UNIT QUANTITY			
Concrete Per Pedestal Thickness	CY/In.	0.040	
Reinforcing Steel	LB	195 (182)	

(The Reinforcing Steel quantity shown in parenthesis is for a Pedestal attached to Pedestrian/Bicycle Railing - Index 521-820 with Bridge Deck or Approach Slab thinner than 1'-11/2". Add 59 Lbs. for Bars 4J1 & 4J2 when Pedestal Thickness is 1'-5½" or greater)



CROSS REFERENCE: For location of Detail "A" see Sheets 1,2 and 3.

TABLE 1 - DESIGN LIMITATIONS FOR							
ANCHOR BOLTS (1" Dia.)							
WIND	ARM	BRIDGE DECK HEIGHT (Ft.)*					
SPEED	LENGTH	DESIGN MOUNTING HEIGHT					
(MPH)	(Ft.)	40 Ft.	45 Ft.	50 Ft.			
130	≤ 15	75	75	75			
150	≤ 15	<i>75</i>	75	75			
170	8 & 10	<i>75</i>	75	45**			
170	12 & 15	75	75	25**			

- \* Above natural ground or MLW.
- \*\* Use  $1\frac{1}{4}$ " diameter Anchor Bolt for Bridge Deck Height greater than shown, in Table 1, up to 75'.

DESCRIPTION: **REVISION** 11/01/21

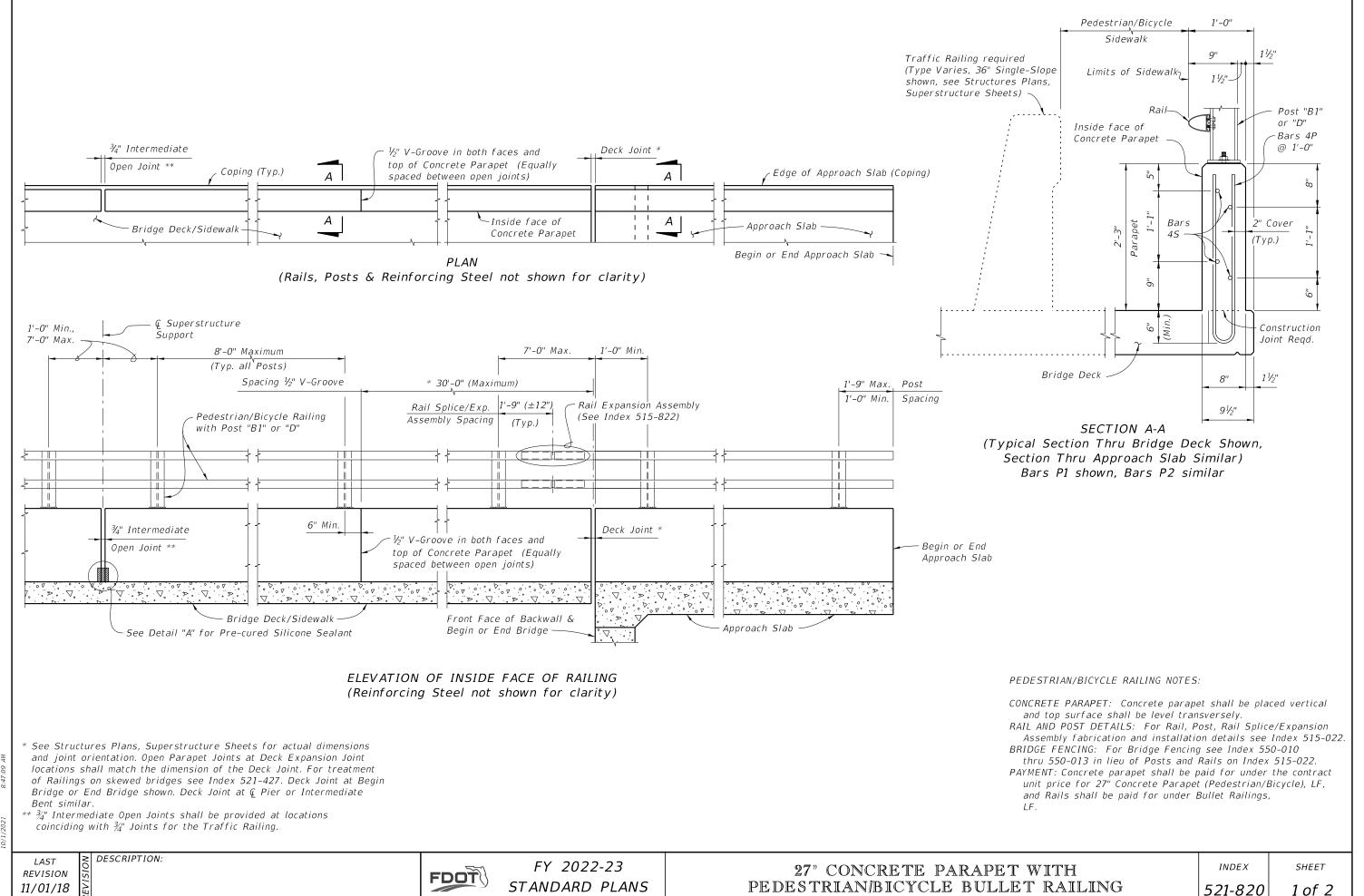


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LIGHT POLE PEDESTAL - BRIDGE

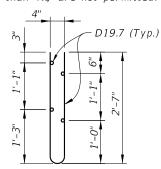
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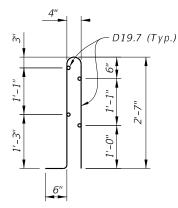
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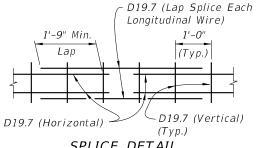
## ALTERNATE REINFORCING (WELDED WIRE REINF.) DETAILS

NOTE: Place wire panels to minimize the end overhang. End Overhangs greater than  $4\frac{3}{4}$ " are not permitted.





## \_\_ WELDED WIRE REINFORCEMENT (WWR) \_\_\_



SPLICE DETAIL (Between WWR Sections)

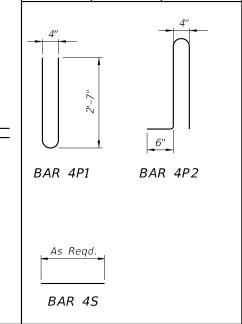
## REINFORCING STEEL NOTES:

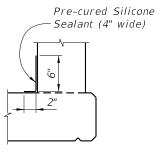
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the parapet on a retaining wall shall be the same as detailed above for a 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Bars 4P2 may be used in lieu of Bars 4P1.
- 6. At the option of the Contractor deformed WWR may be used in lieu of all Bars 4P or 4P2 and 4S.



## BILL OF REINFORCING STEEL

MARK	SIZE	LENGTH
P1	4	5'-6"
P2	4	6'-0"
S	4	As Regd.





## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

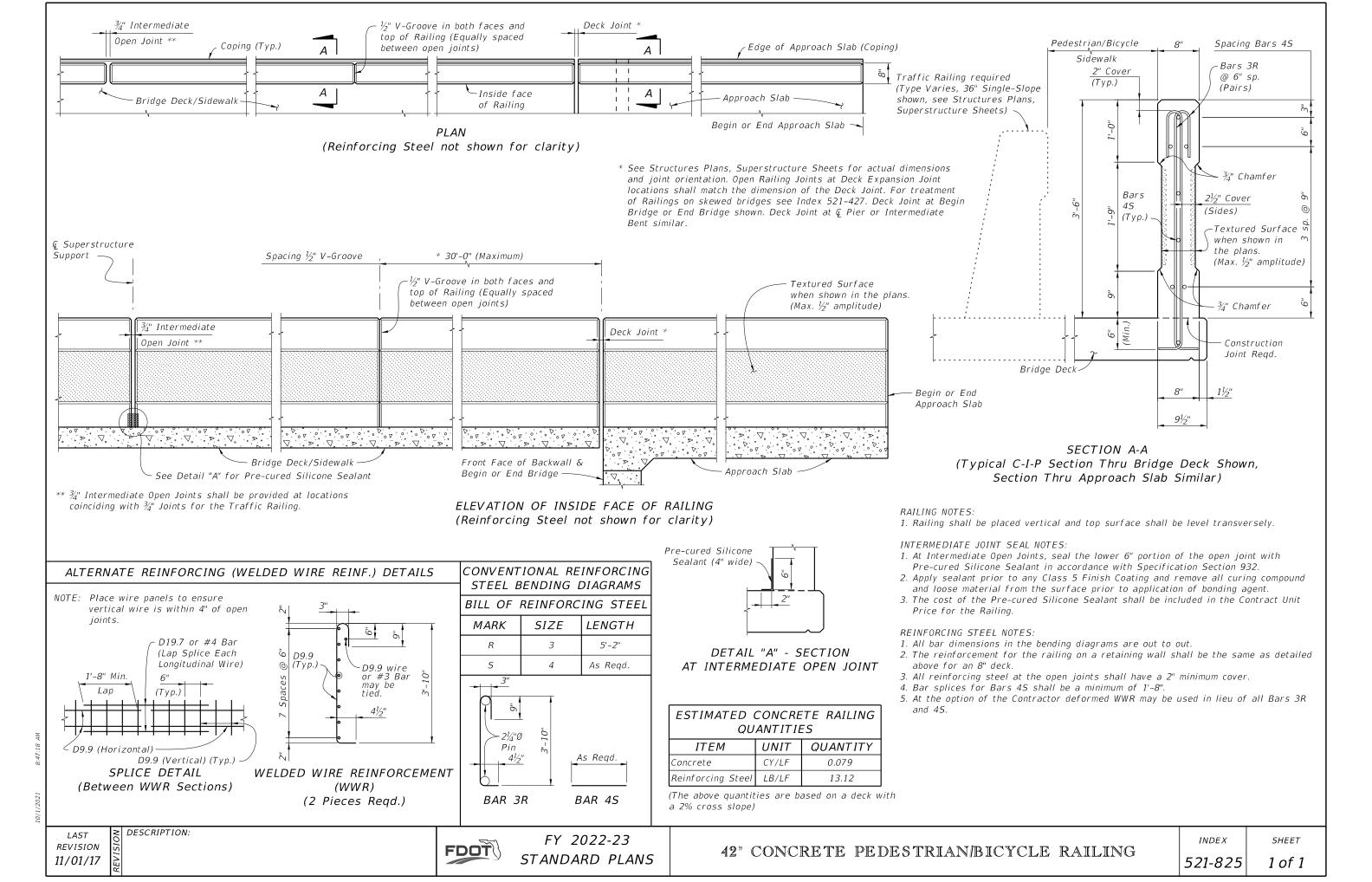
#### INTERMEDIATE JOINT SEAL NOTE:

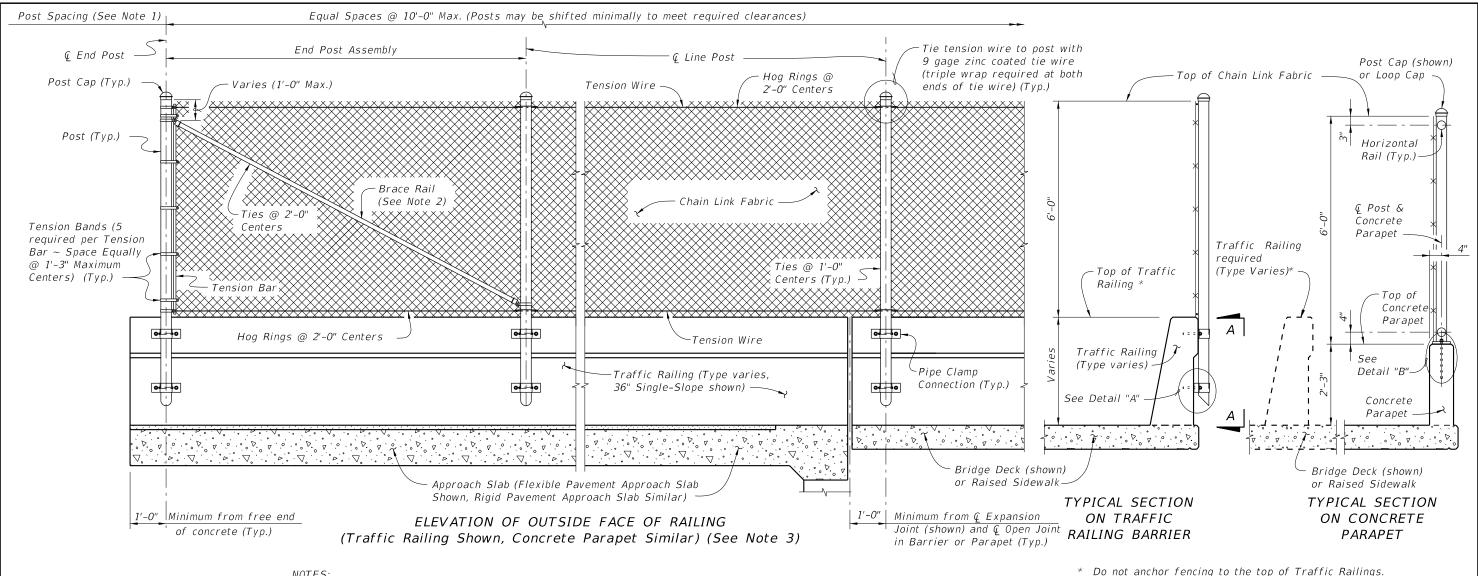
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant meeting the requirements of Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Concrete Parapet.

ESTIMATED CONCRETE PARAPET QUANTITIES					
ITEM	UNIT	QUANTITY			
Concrete	CY/LF	0.056			
Reinforcing Steel (P1 & S)	LB/FT	6.35			
Reinforcing Steel (P2 & S)	LB/FT	6.68			

(The above quantities are based on a deck with a 2% cross slope)

DESCRIPTION:





#### NOTES:

- 1. A Pull Post Assembly is required at maximum intervals of 500'-0". See Sheet 3.
- 2. Brace rails are only required for vertical fence installations on Traffic Railing.
- 3. Provide horizontal rails for vertical fence installations on Concrete Parapets in lieu of tension wire. Locate horizontal rails as shown in the Typical Section for Concrete Parapets at right.

#### FENCING NOTES

#### FENCE INSTALLATION:

Install posts plumb (within a tolerance of  $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

CONCRETE PARAPET DETAILS:

DESCRIPTION:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace rails and bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, post and loop caps, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, bearing pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

#### CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

For Pull Post Assembly Detail for Traffic Railings see Sheet 3.

For Pull Post Assembly Detail for Concrete Parapets and Detail "B" see Sheet 4.

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**FDOT** 

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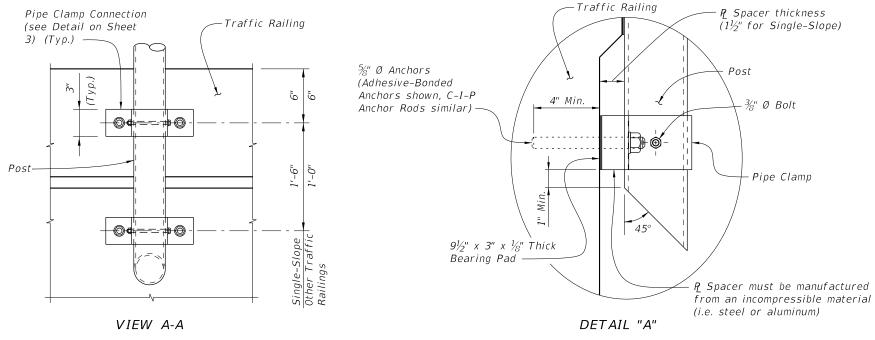
BRIDGE FENCING (VERTICAL)

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TABLE OF CHAIN LINK FENCE COMPONENTS			
COMPONENT ASTM DESIGNATION			COMPONENT INFORMATION
Traffic Railings and Concrete Parapets	Posts	F1083	Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade
	Chain Link Fabric (2" mesh with twisted top and knuckled bottom selvage)	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating
		A491	Aluminum Coated Steel - 9 gage (coated wire diameter)
		F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b
	Tie Wires	F626	Zinc Coated Steel Wire - 9 gage
Traff Conc	Brace Bands	F626	12 Gage (Min. thickness) x ¾" (Min. width) Steel Bands (Beveled or Heavy)
and	Tension Bars	F626	$3_{16}$ " (Min. thickness) x $3_4$ " (Min. width) x 5'-10" (Min. height) Steel Bars
	Tension Bands	F626	14 Gage (Min. thickness) x ¾" (Min. width) Steel Bands
	Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)
	Horizontal Rails	F1083	Galvanized Steel Pipe – $2lac{1}{2}$ " NPS, Schedule 40 Regular Grade
e S	Expansion Rails	F1083	Galvanized Steel Pipe - 2" NPS, Schedule 40 Regular Grade
Concrete Parapets	Bolts	A307	$^{1}\!\!\!/_{\!\!4}$ " Ø x $^{4}\!\!\!/_{\!\!4}$ " Hex Head Bolts for Expansion Rail Connections
Co Pa	Nuts	A563	Hex Nuts for Expansion Rail Connections
	Washers	F 436	Flat Washers for Expansion Rail Connections
Traffic Railings	Tension Wire	A824 & A817	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating
			Type I (Aluminum Coated Steel Wire) - 7 gage
	Hog Rings	F626	Zinc Coated Steel Wire - 12 gage
	Brace Rails	F1083	Galvanized Steel Pipe - 1½" NPS, Schedule 40 Regular Grade

TABLE OF POST ATTACHMENT COMPONENTS			
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION
Pipe Clamps		A36 or A709 Grade 36	½" Steel P_
Base	Plates	A36 or A709 Grade 36	¾" Steel P
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be ¾" Ø
Spacers		-	Plate thickness varies based on traffic railing type (See Detail "A")
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim 5\!\!\!/\!\!\!/~$ Ø x 6" (no spacer) or $5\!\!\!/\!\!\!/~$ Ø x (6" + spacer thickness)
Pipe ( Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)
Base Plate Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim$ $7_8^{\prime\prime}$ Ø x $147_2^{\prime\prime}$
	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections
Washers		F436	Flat Washers for Pipe Clamp and Base Plate Connections
Bearing Pads (Plain Neoprene)		-	In accordance with Specification Section 932 for Ancillary Structures



## ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

## ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

## **WELDING**:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

## CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

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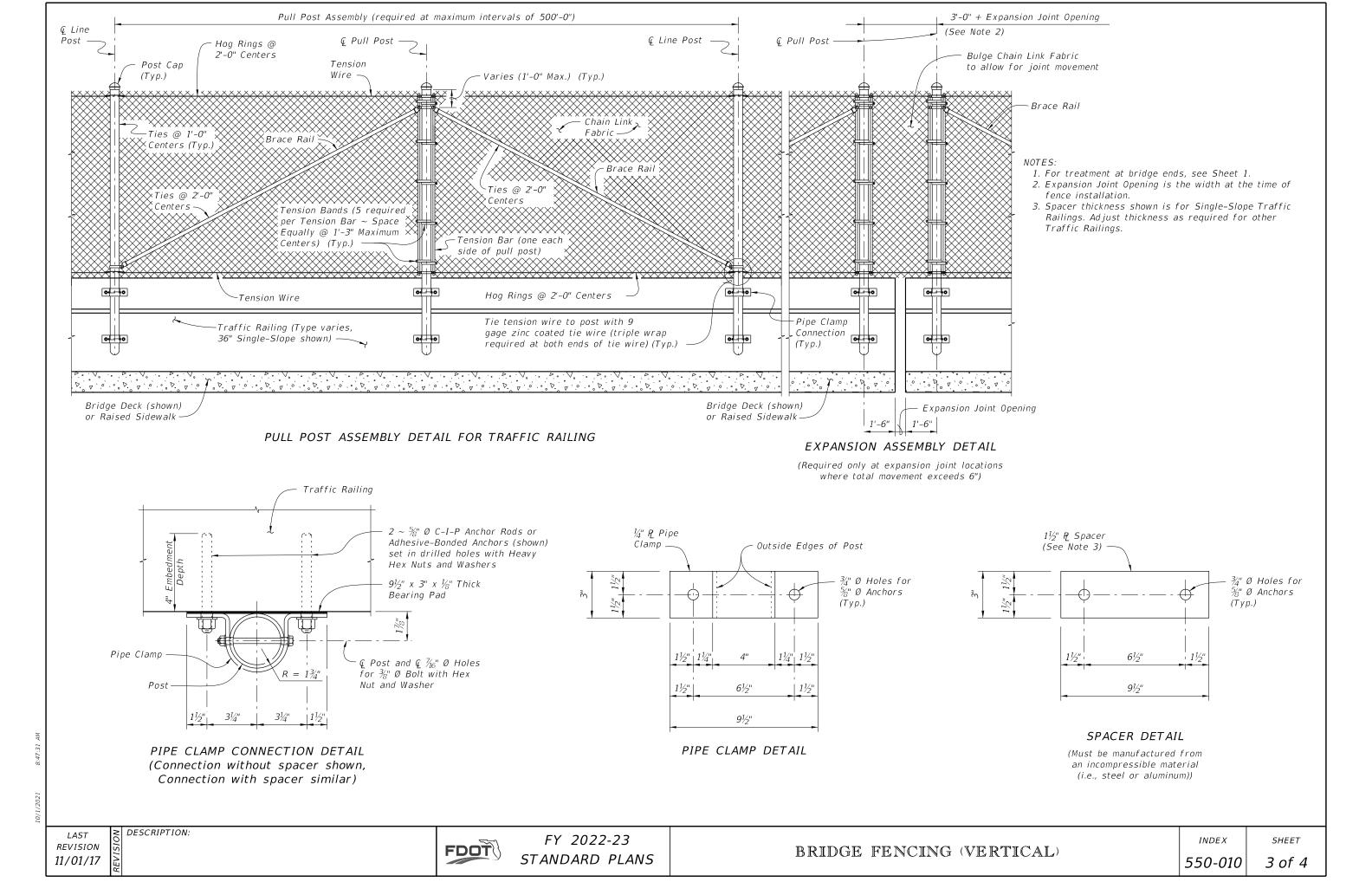
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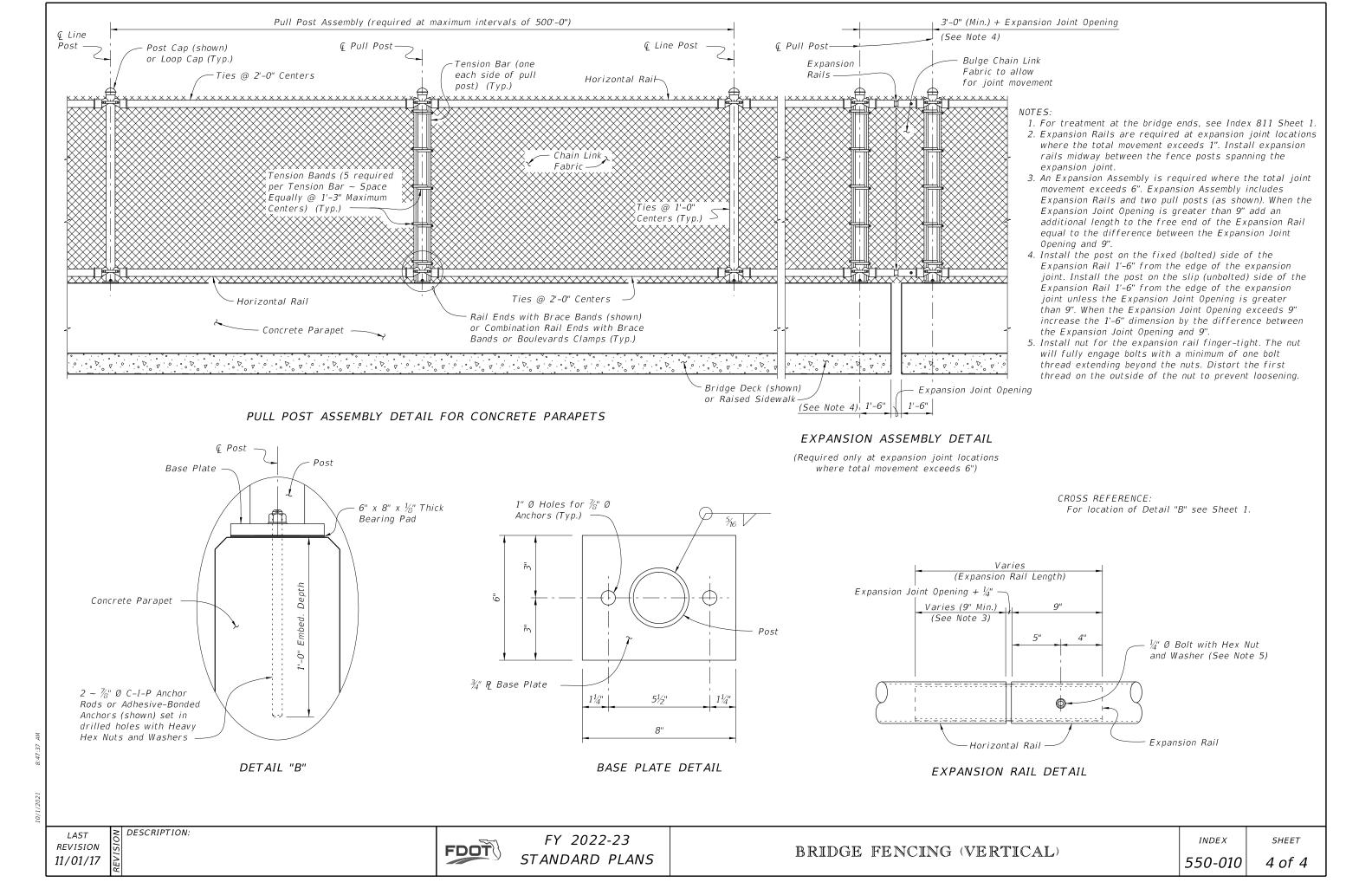
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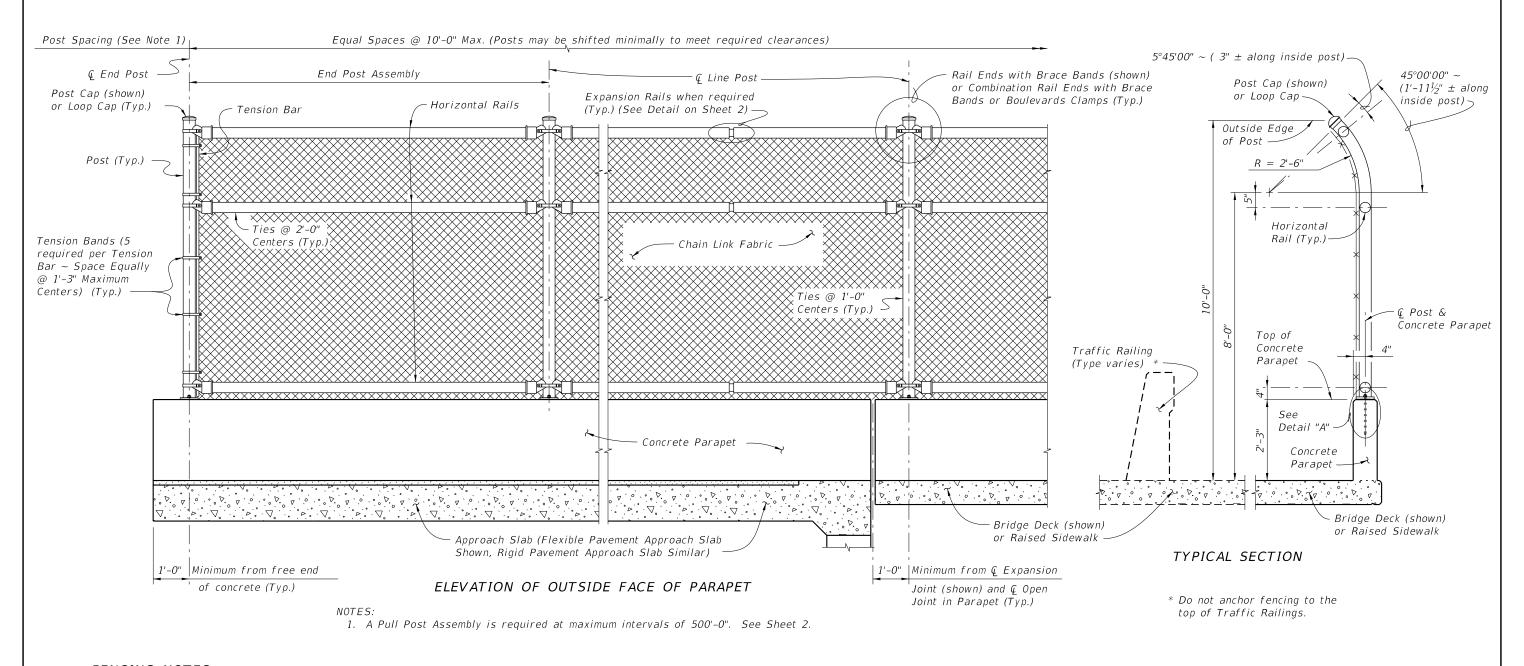
BRIDGE FENCING (VERTICAL)

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# FENCING NOTES

FENCE APPLICATION:

This bridge fence can only be used on sidewalk installations separated from traffic by a traffic railing.

FENCE INSTALLATION:

Install posts plumb (within a tolerance of  $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable.

CONCRETE PARAPET DETAILS:

See Index 521-820 - Pedestrian/Bicycle Bullet Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

DESCRIPTION:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, ties, tension bars and bands, post and loop caps, base plates, anchor rods, bolts, nuts, washers, shim plates, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

#### CROSS REFERENCE:

For Table of Fence Components and Pull Post Assembly Detail see Sheet 2. For Table of Post Attachment Components and Detail "A" see Sheet 3.

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**FDOT** 

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BRIDGE FENCING (CURVED TOP)

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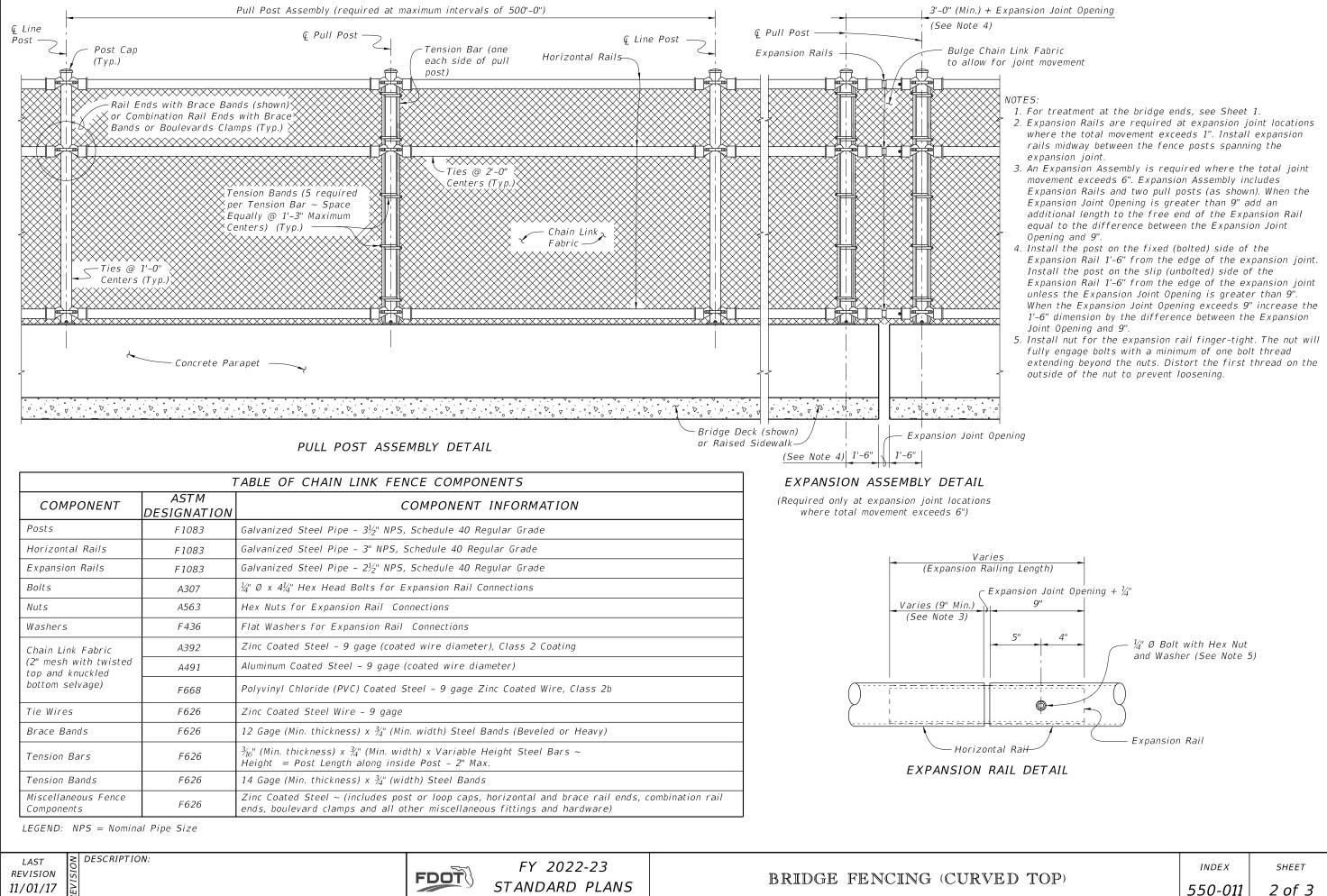


TABLE OF POST ATTACHMENT COMPONENTS			
COMPONENT ASTM DESIGNATION		COMPONENT INFORMATION	
Base Plates	A36 or A709 Grade 36	¾" Steel PL	
A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5		Plate thicknesses as required, Holes in shim plates will be $rac{3}{4}$ " Ø	
Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
C-I-P Anchor Rods F1554 Grade 36		Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
Nuts A563		Hex Nuts for Base Plate Connections	
Washers F436		Flat Washers for Base Plate Connections	
Bearing Pads (Plain)	-	In accordance with Specification Section 932 for ancillary structures	

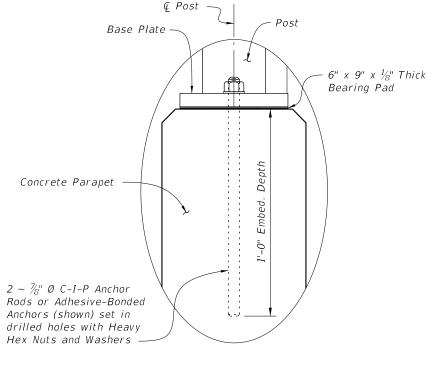
ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

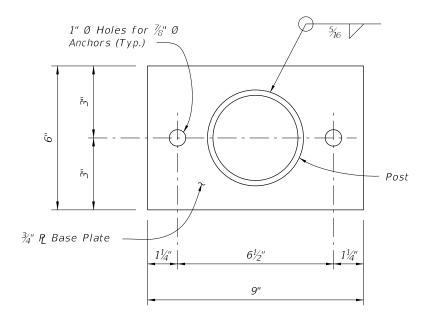
Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates and Base Plates) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication. ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.



DETAIL "A"



BASE PLATE DETAIL

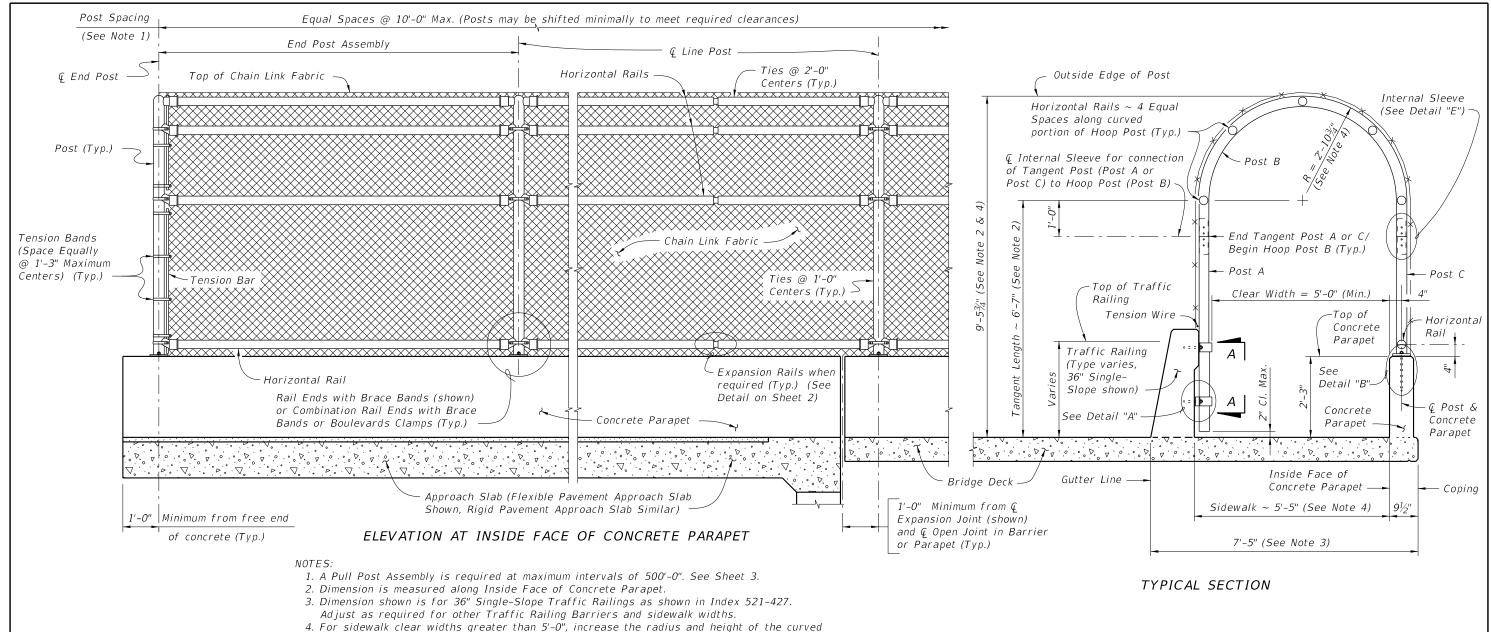
CROSS REFERENCE:

For location of Detail "A" see Sheet 1.

**REVISION** 11/01/17

DESCRIPTION:

FDOT



4. For sidewalk clear widths greater than 5'-0", increase the radius and height of the curved portion of the Hoop Post at the rate of 6" for every one foot increase in sidewalk width.

# FENCING NOTES

## FENCE INSTALLATION:

Install posts plumb (within a tolerance of  $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable.

## TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

### CONCRETE PARAPET DETAILS:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

## LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

#### PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

#### CROSS REFERENCE:

For Table of Fence Components and Table of Post Attachment Components see Sheet 2. For Pull Post Assembly Detail, View A-A and Detail "A" see Sheet 3. For Detail "B" and "E" see Sheet 4.

DESCRIPTION: **REVISION** 11/01/17

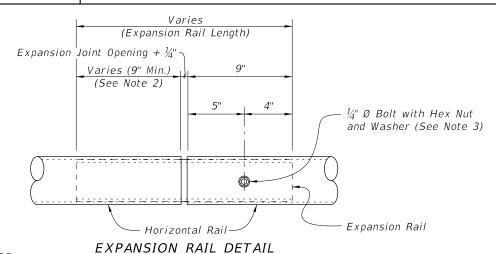
**FDOT** 

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- 1. Expansion Rails are required at expansion joint locations where the total movement exceeds 1". Install expansion rails midway between the fence posts spanning the expansion joint.
- 2. An Expansion Assembly is required where the total joint movement exceeds 6". Expansion Assembly includes Expansion Rails and two pull posts (see Sheet 3). When the Expansion Joint Opening is greater than 9" add an additional length to the free end of the Expansion Rail equal to the difference between the Expansion Joint Opening and 9".
- 3. Install nut for the expansion rail finger-tight. The nut will fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening.

	TABLE OF POST ATTACHMENT COMPONENTS			
	COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe	Clamps	A36 or A709 Grade 36	¼" Steel ዊ	
Base	Plates	A36 or A709 Grade 36	¾" Steel P	
Shim	Plates	A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $\frac{3}{4}$ " Ø	
Spac	ers	-	Plate thickness varies based on Traffic Railing type. (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Base Plate Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim$ $7_8^{\circ}$ 0 x $14^{1}\!\!/_{2}^{\circ}$	
Base Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim  \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
Bolts	5	A307	3%" Ø x 434" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections	
Wash	ners	F 436	Flat Washers for Pipe Clamp and Base Plate Connections	
Bear	ing Pads (Plain)	-	In accordance with Specification Section 932 for Ancillary Structures	

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

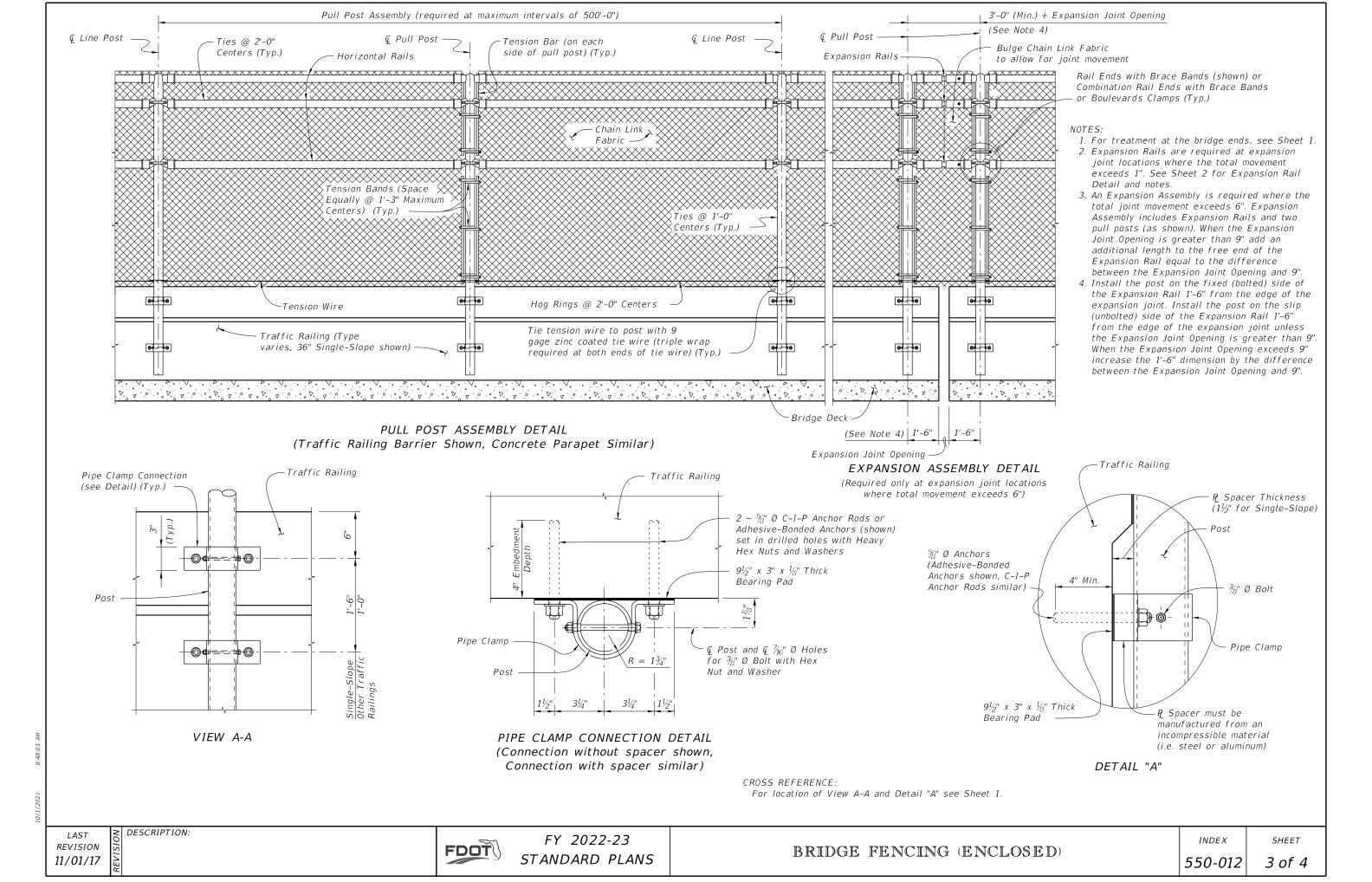
Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

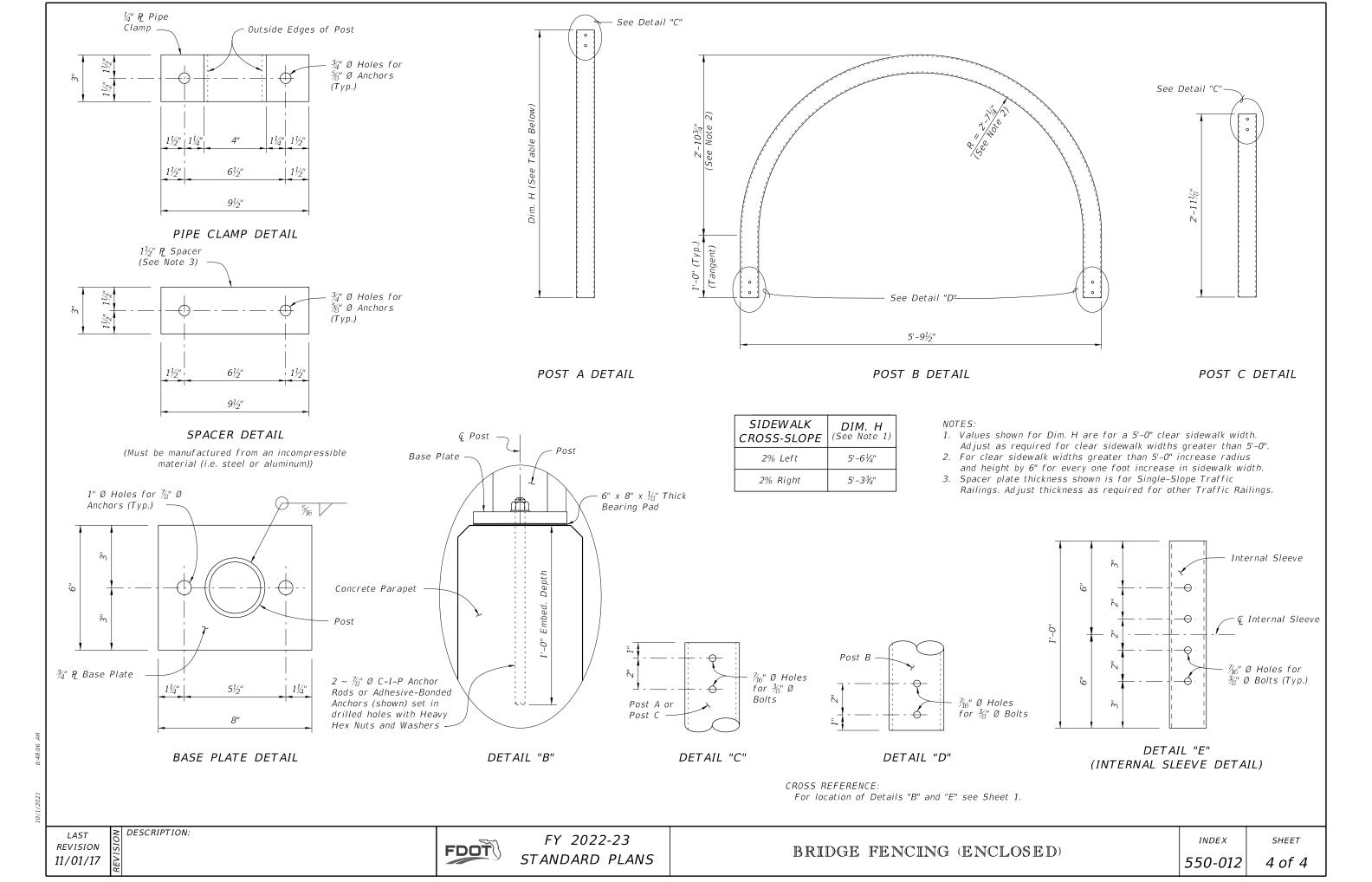
ADHESIVE-BONDED ANCHORS AND DOWELS:

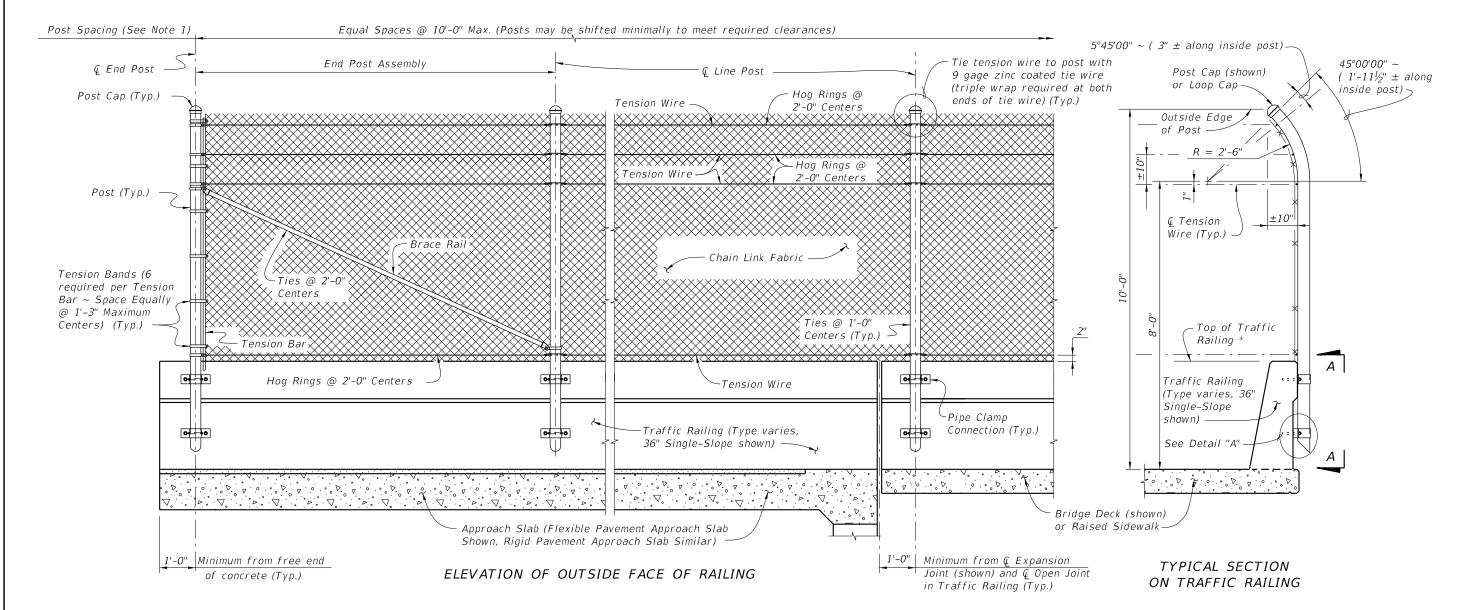
Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

DESCRIPTION:







NOTES:

1. A Pull Post Assembly is required at maximum intervals of 500'-0". See Sheet 3.

\* Do not anchor Fencing to the top of Traffic Railings.

## FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of  $\pm$   $1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

LIMITS OF FENCING:

DESCRIPTION:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes all materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

For Pull Post Assembly Detail for Traffic Railing see Sheet 3.

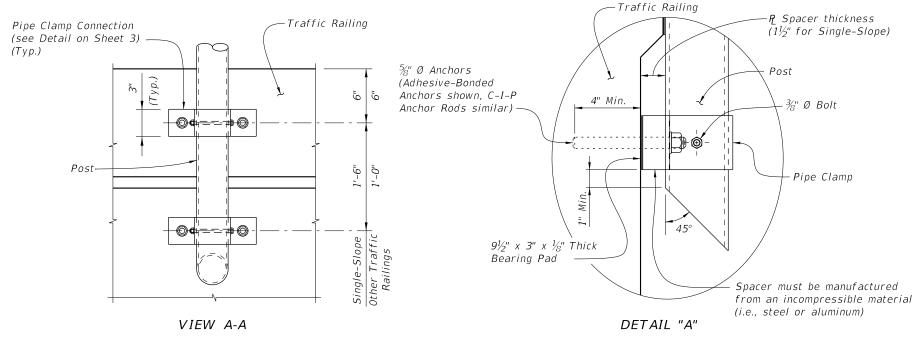
LAST **REVISION** 11/01/17



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TABLE OF CHAIN LINK FENCE COMPONENTS			
COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION	
Posts	Posts F1083 Galvanized Steel Pipe - 3½" NPS, Schedule 40 Regular Grade		
Chain Link Fabric (2" mesh with twisted	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating	
top and knuckled bottom selvage)	A491	Aluminum Coated Steel - 9 gage (coated wire diameter)	
	F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b	
Tie Wires	F626	Zinc Coated Steel Wire - 9 gage	
Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)	
Tension Bars	F626	$\frac{3}{16}$ " (Min. thickness) x $\frac{3}{4}$ " (Min. width) x 6'-10" (Min. height) Steel Bars	
Tension Bands	F626	14 Gage (Min. thickness) x ¾" (Min. width) Steel Bands	
Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)	
Tanalan Mina	A824 & A817	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating	
Tension Wire		Type I (Aluminum Coated Steel Wire) - 7 gage	
Hog Rings	F626	Zinc Coated Steel Wire - 12 gage	
Brace Rails	F1083	Galvanized Steel Pipe – $1\frac{1}{4}$ " NPS, Schedule 40 Regular Grade	

	TABLE OF POST ATTACHMENT COMPONENTS			
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe	Clamps	A36 or A709 Grade 36	¼" Steel ዊ	
Base	Plates	A36 or A709 Grade 36	¾" Steel P	
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $34^{\circ}$ Ø	
Space	ers	-	Plate thickness varies based on traffic railing type (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Pipe ( Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp Connections	
Wash	ers	F 436	Flat Washers for Pipe Clamp Connections	
Bearing Pads (Plain Neoprene)		-	In accordance with Specification Section 932 for Ancillary Structures	



## ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

# ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

# **WELDING**:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

## CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

REVISION 11/01/20

DESCRIPTION:

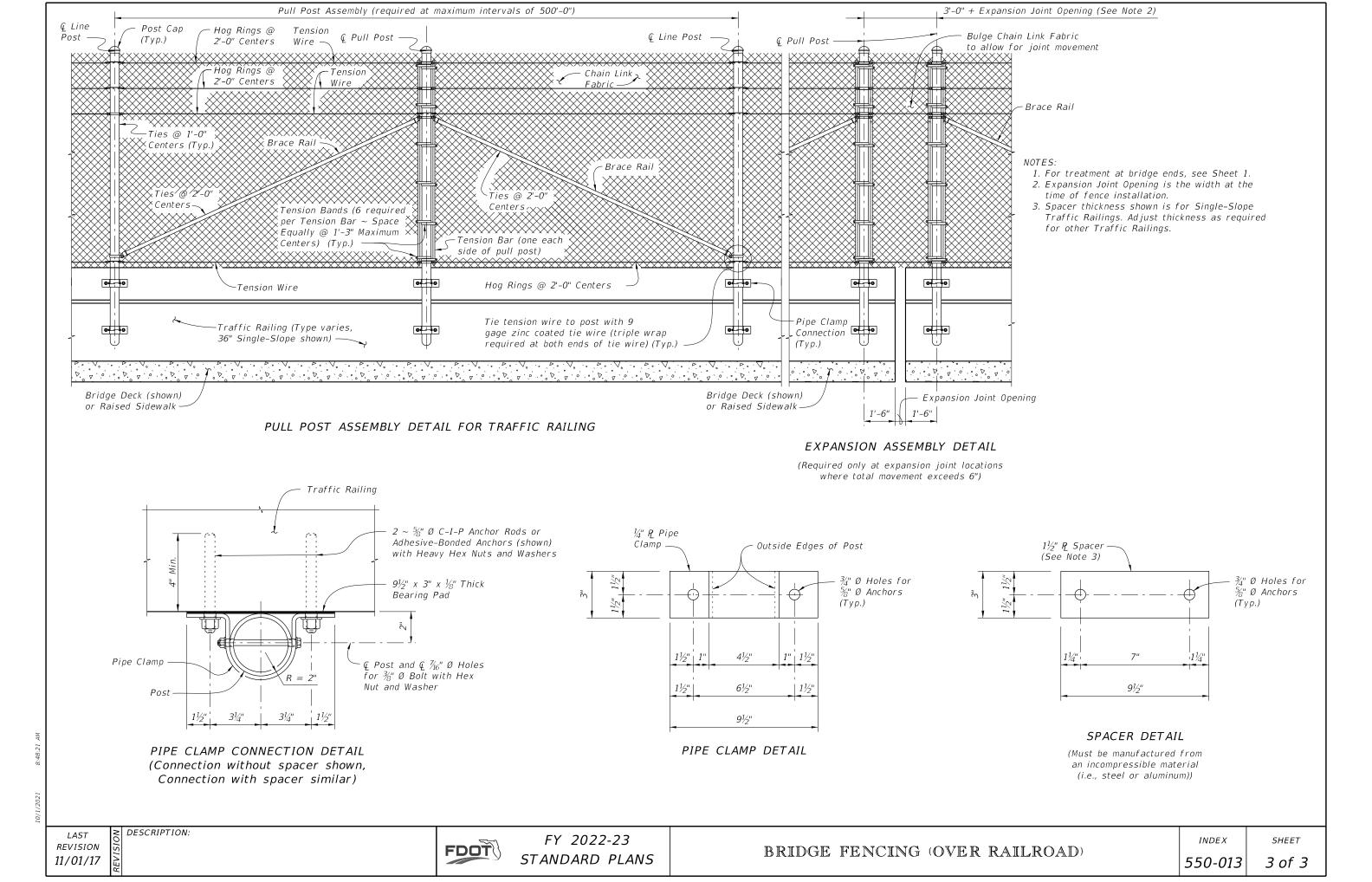
FDOT

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BRIDGE FENCING (OVER RAILROAD)

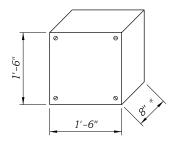
INDEX 550-013

SHEET 2 of 3

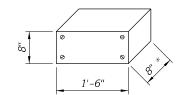


- 1. Furnish and install approved Conduits, Fittings and Embedded Junction Boxes (EBJ's) in accordance with Specification Sections 630 and 635, this Standard, the National Electric Code (NEC) and as directed by the Engineer.
- 2. Furnish and install Embedded Junction Boxes (EJB) with weatherproof covers sized in accordance with NEC requirements and the maximum size limits shown. Install EJB adjacent to the Begin and End of Bridges, Begin and End of Retaining Walls, (except omit EJB adjacent to the Bridge unless a precast Traffic Railing with junction slab is used), and at other locations as necessary to maintain 300 foot maximum spacing. See Plans for additional locations and details.
- 3. For Conduit not designated for future use, see Plans for details. For Conduit designated for future use, stub out and cap the Conduit. Drive a 3'-0" $\pm$  long  $\frac{3}{4}$ " (min.) diameter Steel Pipe flush with the ground line adjacent to the end of the Conduit as shown on Sheets 2, 3 or 4. Provide the location of the stub out with Steel Pipe to the Engineer for inclusion on the As-Built Plans.
- 4. Shift vertical Railing reinforcement symmetrically to provide 2" clearance to EJB. Space shifted vertical reinforcement at minimum 3" centers. Cut horizontal Railing reinforcement to provide 2" clearance to EJB and provide supplemental reinforcement as shown. To facilitate placement of Conduit, Expansion Fittings, and Expansion/Deflection Fittings, shift reinforcing a maximum of 1" but do not cut railing reinforcing to facilitate Conduit or Fittings. Do not bundle Conduits, or Conduit and horizontal reinforcement.
- 5. Place conduits as indicated in this Standard unless Structures Plans indicate fewer.

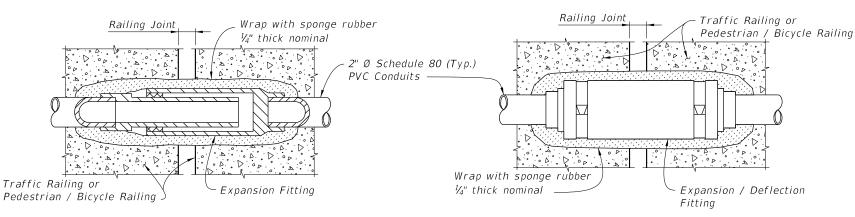
\* Reduce to 6" maximum when installed in Pedestrian/ Bicycle Railings.



EJB "A" Double or Triple Conduit (Maximum Dimensions)



EJB "B" Single Conduit (Maximum Dimensions)



DETAIL "A" EXPANSION FITTING DETAIL

DETAIL "B" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / CONCRETE) DETAIL "C" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / SOIL)

**GENERAL** 

**REVISION** 11/01/20

DESCRIPTION:

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(Min.)

<u>3</u>

Expansion / Deflection

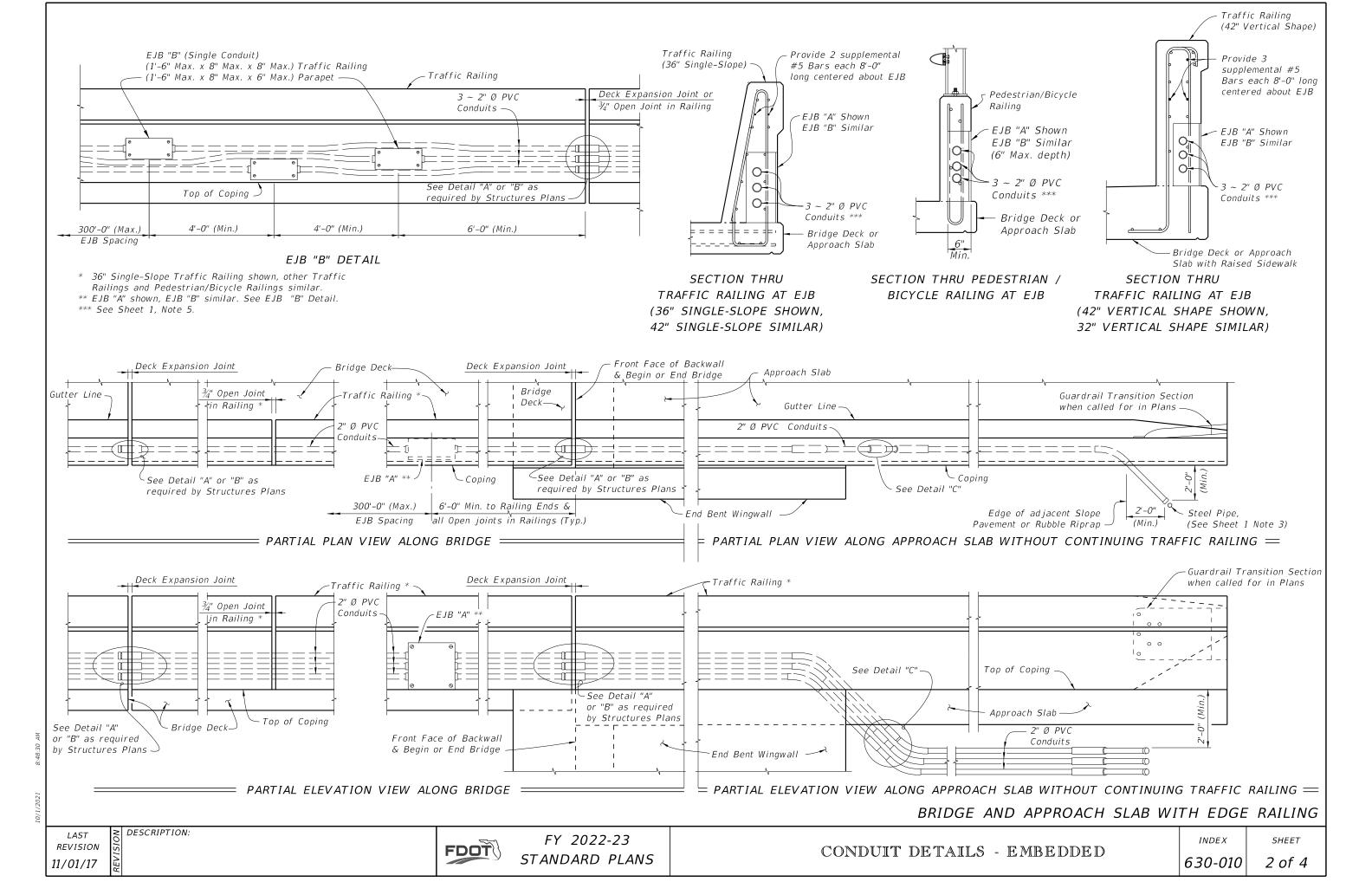
INDEX 630-010

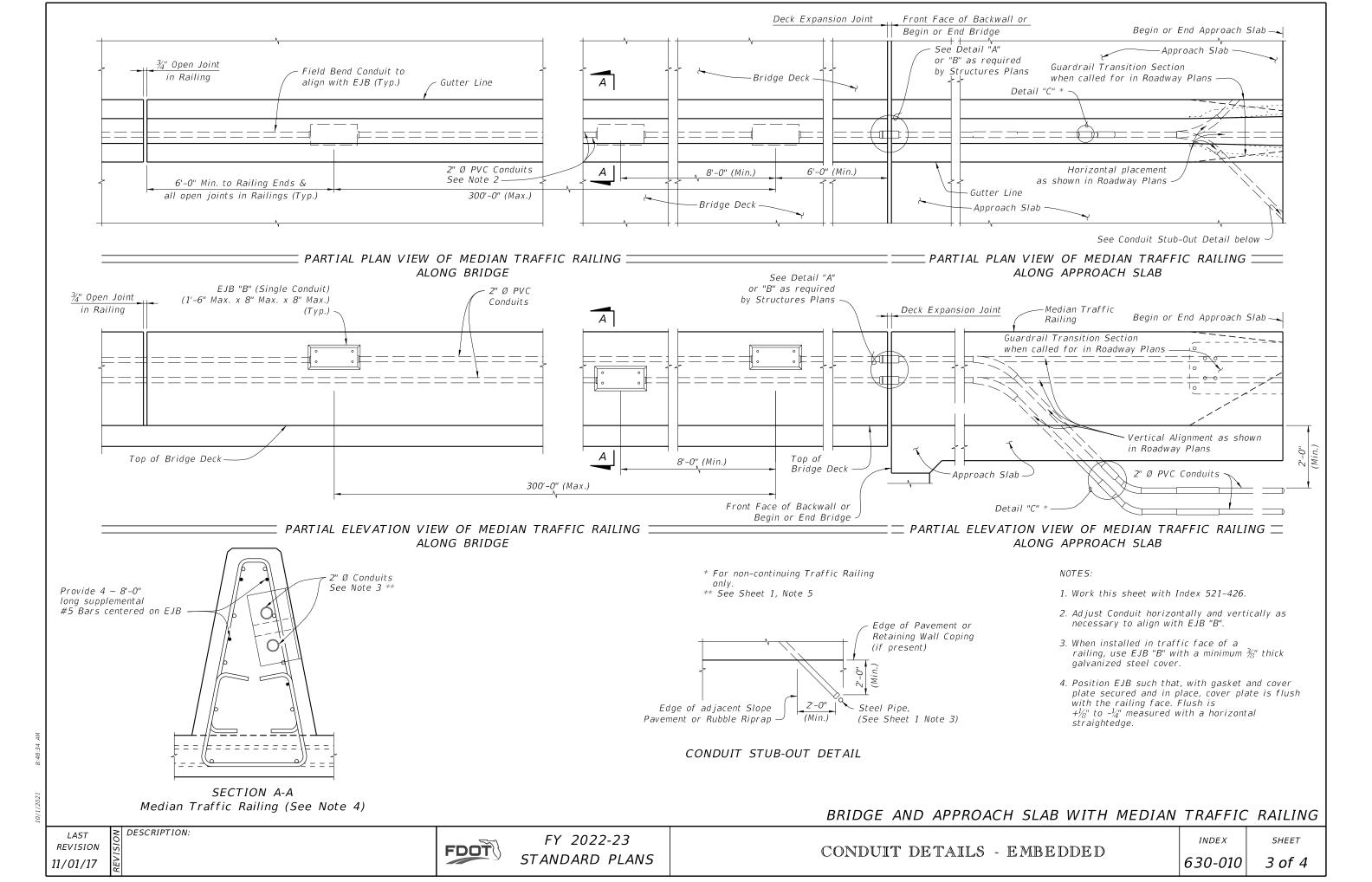
Wrap with sponge rubber

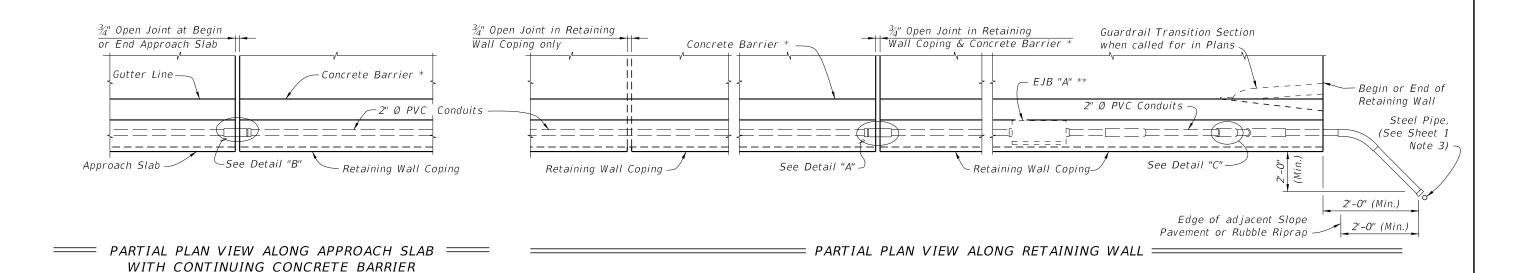
' Ø PVC Conduits

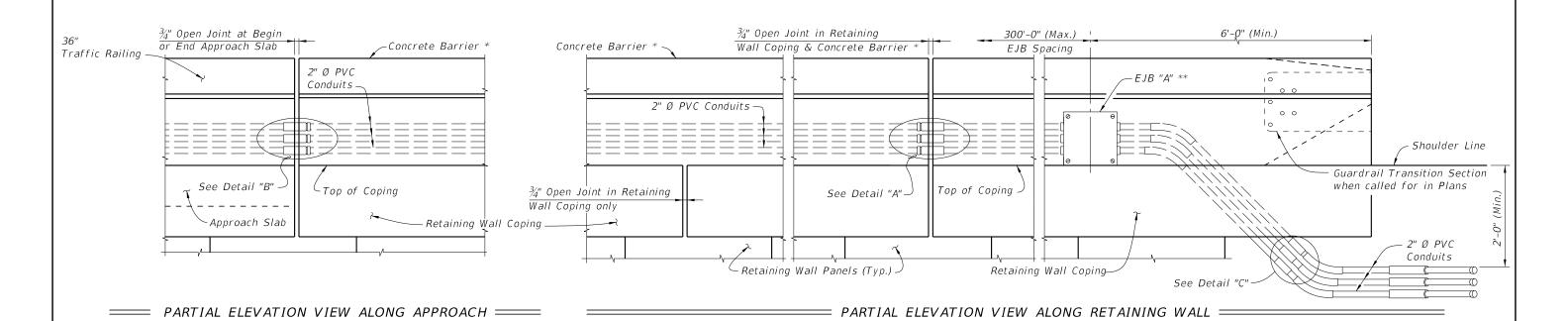
1/3" thick nominal

SHEET 1 of 4









SLAB WITH CONTINUING Concrete Barrier (Retaining Wall Mounted Concrete Barrier shown, Traffic Railing similar)

- \* Index 521-610 Concrete Barrier/Junction Slab shown, other railings and parapets similar.
- \*\* EJB "A" shown EJB "B" similar. See EJB "B" Detail on Sheet 2.

APPROACH SLAB AND RETAINING WALL WITH CONCRETE BARRIER

**REVISION** 11/01/18

DESCRIPTION:

FDOT

FY 2022-23 STANDARD PLANS

CONDUIT DETAILS - EMBEDDED

INDEX 630-010

SHEET 4 of 4

#### **BOX GIRDER MAINTENANCE LIGHTING NOTES:**

- 1. Submit shop drawings to the Engineer detailing the layout of the maintenance lighting system for the entire structure. The shop drawings must include, but not be limited to, the following items:
  - a. Conduit layout and installation details through diaphragms, around post-tensioning (PT) ducts, lateral bracing and cross frames as necessary.
  - b. Conduit access through box girder end diaphragms with minimum 1" clearance in all directions.
  - c. Conduit expansion fitting details.
  - d. Fastener details for the interior electrical system.
  - e. Single line diagram showing mini power centers, switches, contactors, timers, etc.
  - f. Mini power center details including circuit breaker details.
  - g. Mini power center mounting details if required.
  - h. Feeder schedule.
- 2. Ensure installation meets all requirements of the latest edition of the National Electrical Code (NEC) and local ordinances. Install grounding in accordance with NEC Article 250. Maintain separation between 480V and 120V Conductors / Conduits
- 3. Furnish all labor, equipment, materials, and incidentals required for a complete and functional installation.
- 4. Use only new, unused and Underwriters Laboratories (UL) listed equipment and materials for outdoor use.
- 5. Furnish and install polyvinyl chloride (PVC) conduit in conformance with UL Section 651, NEC Section 347 and NEMA TC-2, UV-resistant and schedule 80. Bend conduits as necessary to connect to loads.
- 6. Provide PVC sleeve 2" larger in diameter than conduit to accommodate construction tolerance.
- 7. Install a UL labeled expansion fitting for specified PVC conduit at all structure expansion joints. Provide certification that the expansion fitting meets the following minimum requirements: Compatibility with the connected conduits, waterproof, UV protected and allows longitudinal movement equal to that of the Expansion Joint.
- 8. Use only Alloy 316 stainless steel supporting hardware. Provide minimum  $\frac{3}{6}$  of fasteners. For concrete or SIP form mounting, provide anchor bolts (expansion, drop-in or adhesive) suitable for dynamic loading (due to vibration caused by traffic). Install fasteners to avoid conflicts with reinforcing steel and PT ducts. For structural steel mounting, do not attach fasteners to main members, i.e. webs and flanges.
- 9. Furnish power distribution at 480V AC. 1 phase, with step down transformers at regular intervals. Furnish 7.5 KVA mini power center with eight 20A breakers as the step down transformer, feeding a maximum of 20 lamps and 20 receptacles. Each mini power center will provide power to no more than 1000' of bridge, preferably 500' on each side of the mini power center. 480V top feed, 120V bottom feed to maintain separation.
- 10. Furnish and install lighting contactors to switch the 480V AC feeding the mini power centers.
- 11. Furnish and install copper conductors, Type XHHW. Do not use any conductor larger than #4 AWG.
- 12. Provide enough slack in all interior cable terminations to allow for minor shifting of the structure.
- 13. Furnish and install National Electric Manufacturers Association (NEMA) Type 4X (non-metallic) surface mounted boxes sized in
- 14. Furnish and install 120V duplex receptacles (GFI, NEMA Type 5-20R), in non-metallic outlet boxes at 50' maximum on centers. Provide each receptacle with a gasketed weather-protective outdoor plate. Maximum wire size to connect to receptacles is #12 AWG.
- 15. Furnish and install surface mounted, fully enclosed, incandescent light fixtures with gasketed clear globes and wire guards at 50' maximum on centers. Provide 100 watt, 130 volt, vibration resistant and brass base incandescent lamps.
- 16. Provide six hour reset timers for each circuit to turn off the lighting system automatically.

CROSS REFERENCES:

- 1. For Maintenance Light Details, see Sheet 2.
- 2. For actual bridge section, see Structures Plans.

DESCRIPTION:

