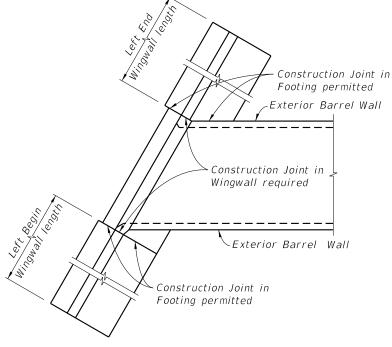


a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic "B".



PART PLAN SHOWING PARALLEL WINGWALLS AND LOCATION OF CONSTRUCTION JOINTS

NOTE:

DESCRIPTION:

Construction Joints in wingwalls and footings are located as follows: For non-skewed wingwalls they are located adjacent to the exterior face of the exterior barrel wall; when the Ç of wingwall and Ç of exterior barrel wall results in an acute angle see Left End Wingwall above, and when the angle is obtuse see Left Begin Wingwall above and Detail C (Sheet 5).

GENERAL NOTES:

LIVE LOAD: HL-93.

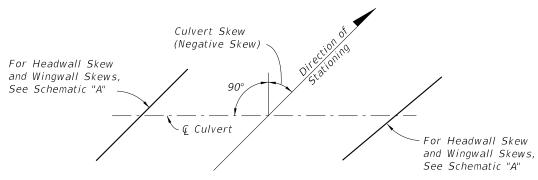
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 8.

REINFORCING STEEL: See the "Box Culvert Data Tables" in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.

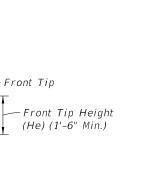


SCHEMATIC "B" - PLAN VIEW CULVERT ALIGNMENT

NOTE: For Culvert Skew see Contract Plans.

TABLE 1 - MINIMUM BAR SPLICE LENGTHS													
FOR LONGITUDINAL REINFORCING													
SPLICE (CLASS B)	BAR	SPLICE (CLASS B)									
CLASS II CLASS IV SIZE CLASS II CLASS IV													
(3400 psi) (5500 psi) (3400 psi) (5500 psi)													
1'-4"	1'-0"	#8	3'-5"	2'-8"									
1'-9"	1'-4"	#9	4'-3"	3'-4"									
2'-2"	1'-8"												
2'-7"	2'-0"												
3'-0"	2'-4"												
(SPLICE (CLASS II B400 psi) 1'-4" 1'-9" 2'-2" 2'-7"	SPLICE (CLASS B) CLASS II CLASS IV 8400 psi) (5500 psi) 1'-4" 1'-0" 1'-9" 1'-4" 2'-2" 1'-8" 2'-7" 2'-0"	SPLICE (CLASS B) BAR SLASS II CLASS IV SIZE 3400 psi) (5500 psi) 1'-4" 1'-0" #8 1'-9" 1'-4" #9 2'-2" 1'-8" 2'-7" 2'-0"	SPLICE (CLASS B) BAR SPLICE (CLASS II CLASS IV SIZE CLASS II 3400 psi) (5500 psi) (3400 psi) 1'-4" 1'-0" #8 3'-5" 1'-9" 1'-4" #9 4'-3" 2'-2" 1'-8" 2'-0"									

TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.



END ELEVATION OF CULVERT

Construction Joint

(See Detail "F",

Sheet 5)

Limits of sloped

top surface (Lw)

_ _ _ _ _ _ _ _ _ _ _ _ _

Half Elevation showing

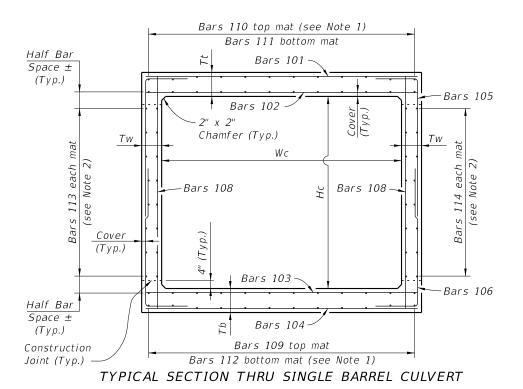
Tapered Wingwalls

REVISION 11/01/16 Front Tip

Half Elevation showing

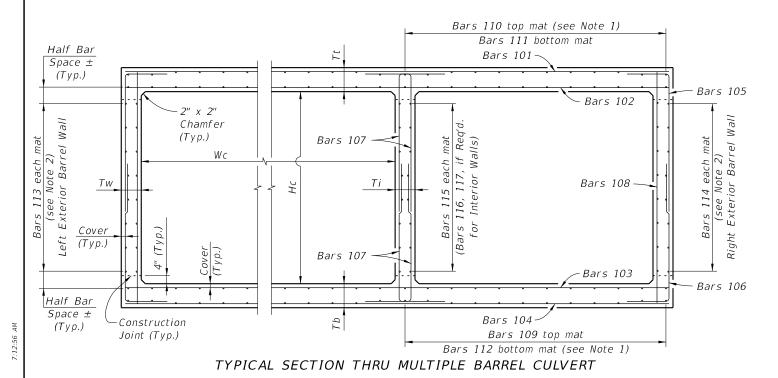
Parallel Wingwalls

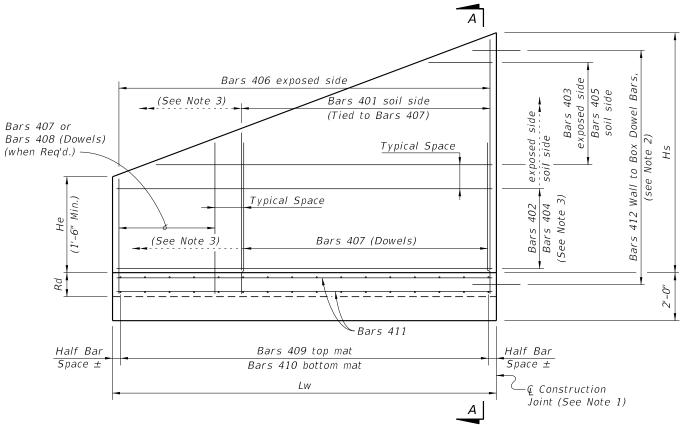
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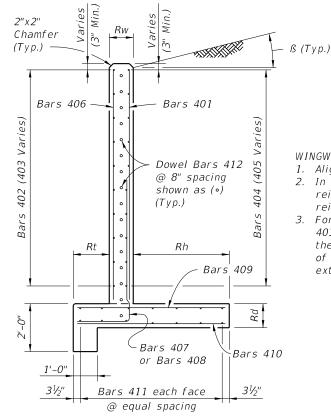
CULVERT BARREL NOTES:

- 1. Space Bars 110 and 112 with a bar in each corner, and at the $\c Q$ of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4" clear.





WINGWALL ELEVATION - Variable Height (Left End shown - other corners similar)



WINGWALL NOTES:

- 1. Align construction joint perpendicular to wingwall.
- 2. In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover.
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

WINGWALL SECTION A-A

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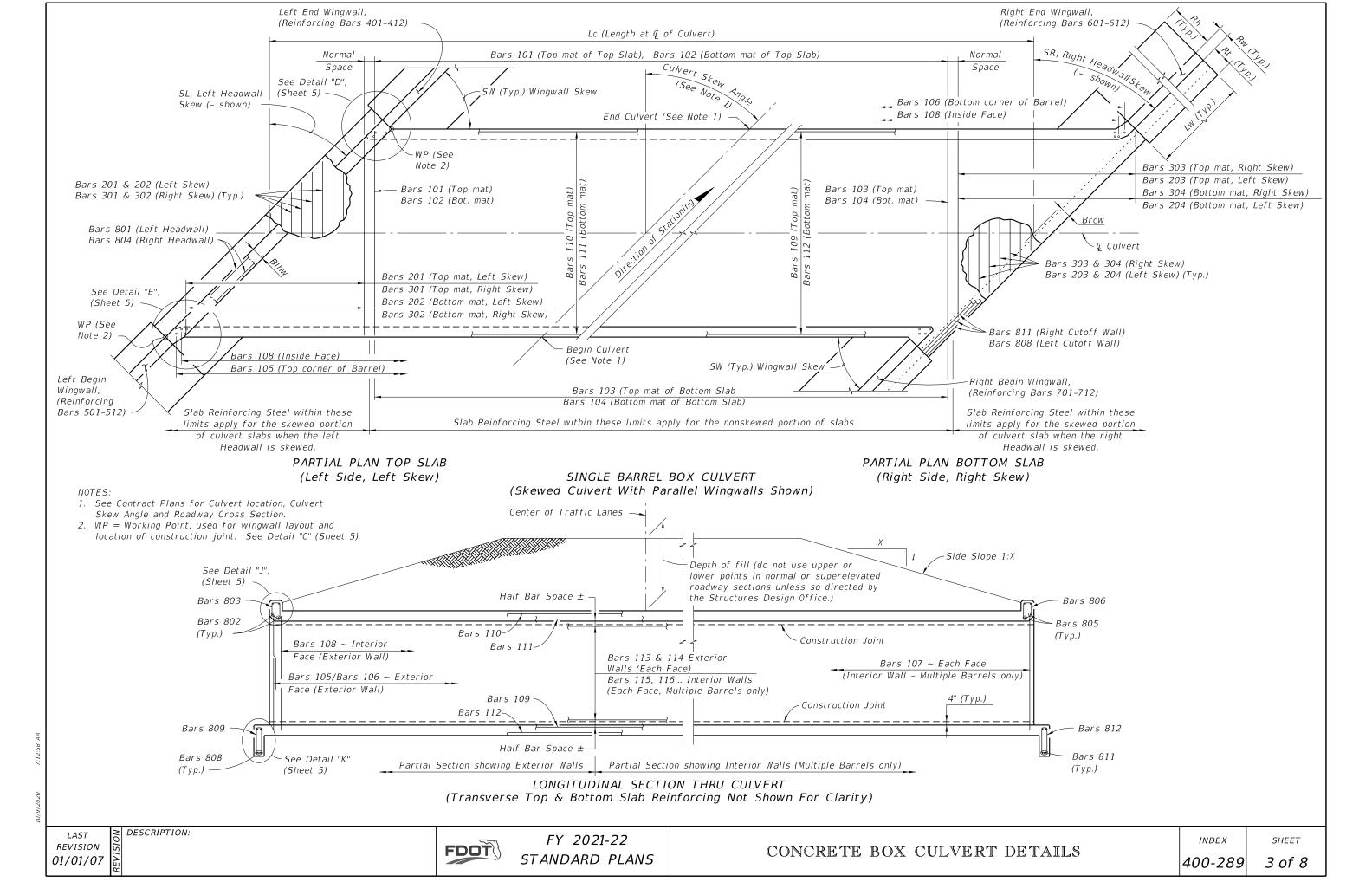
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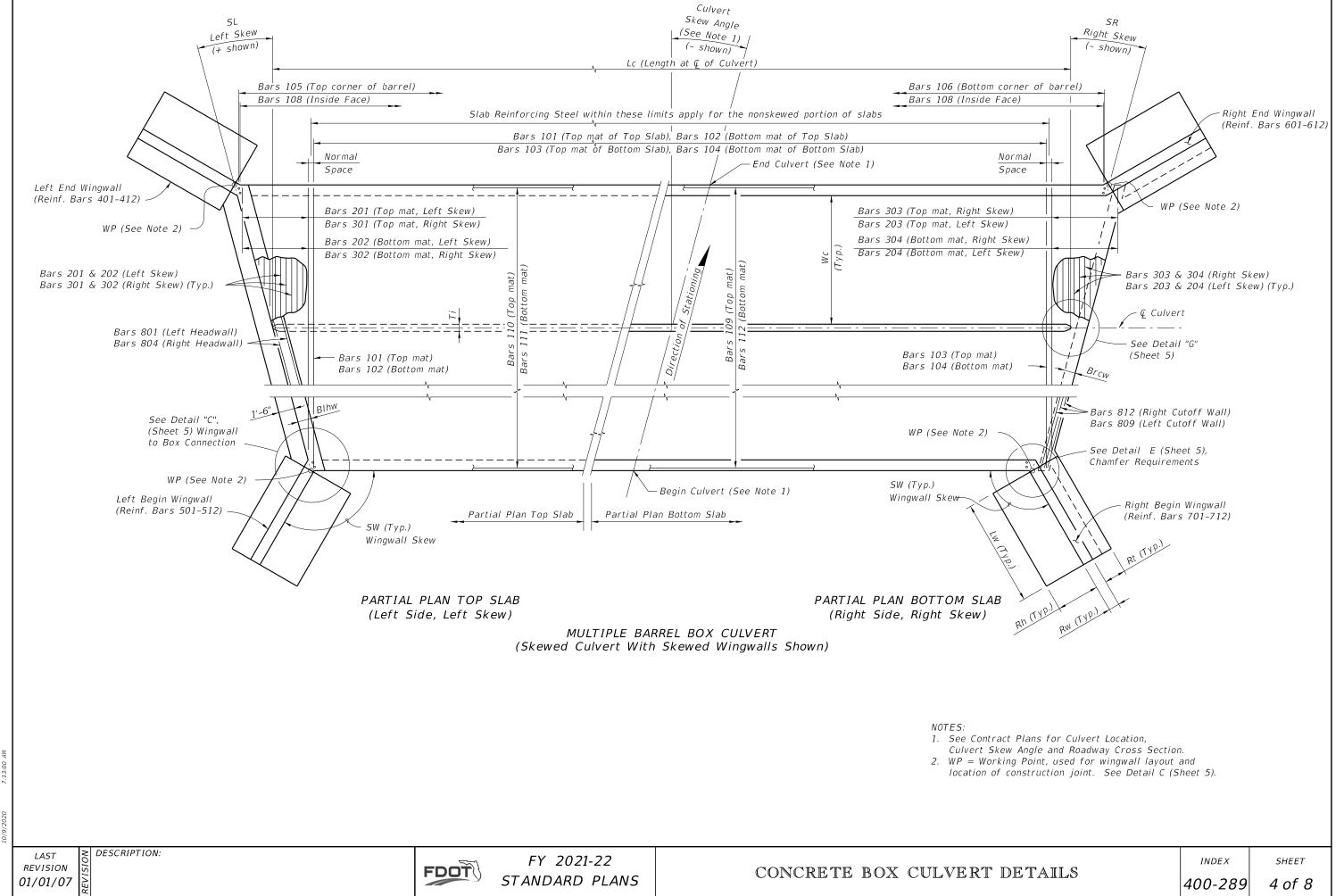
FDOT

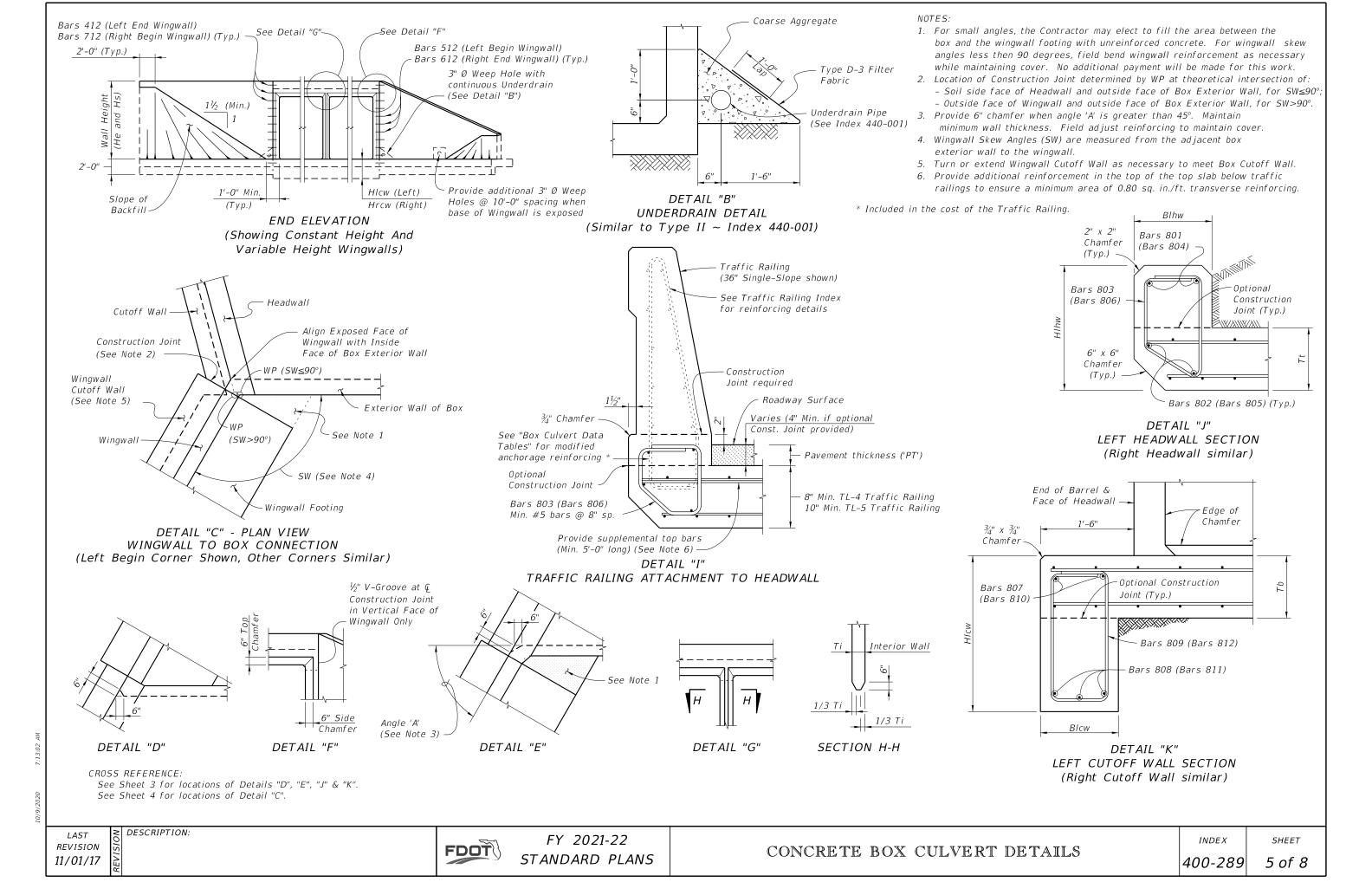
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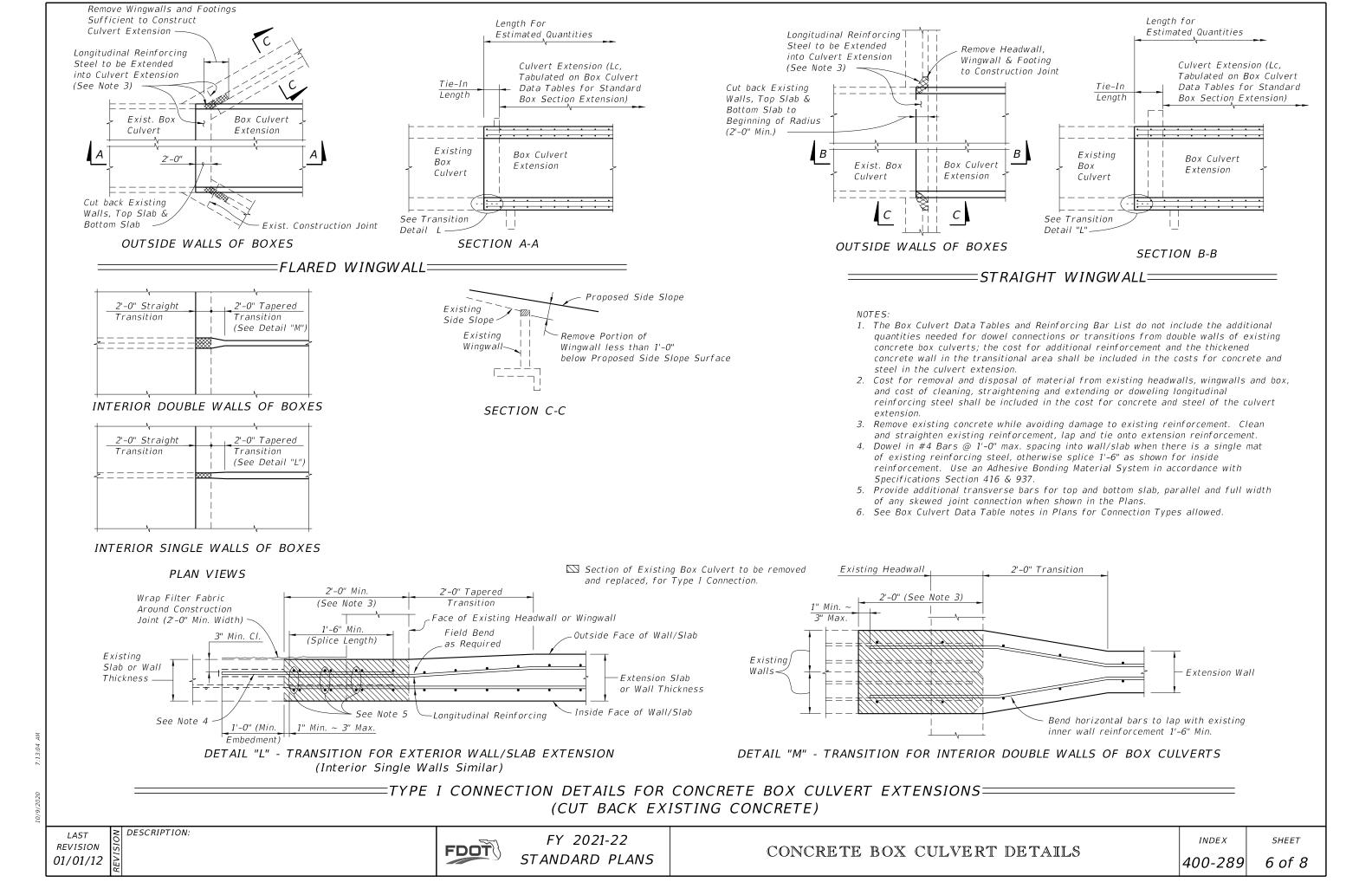
INDEX 400-289

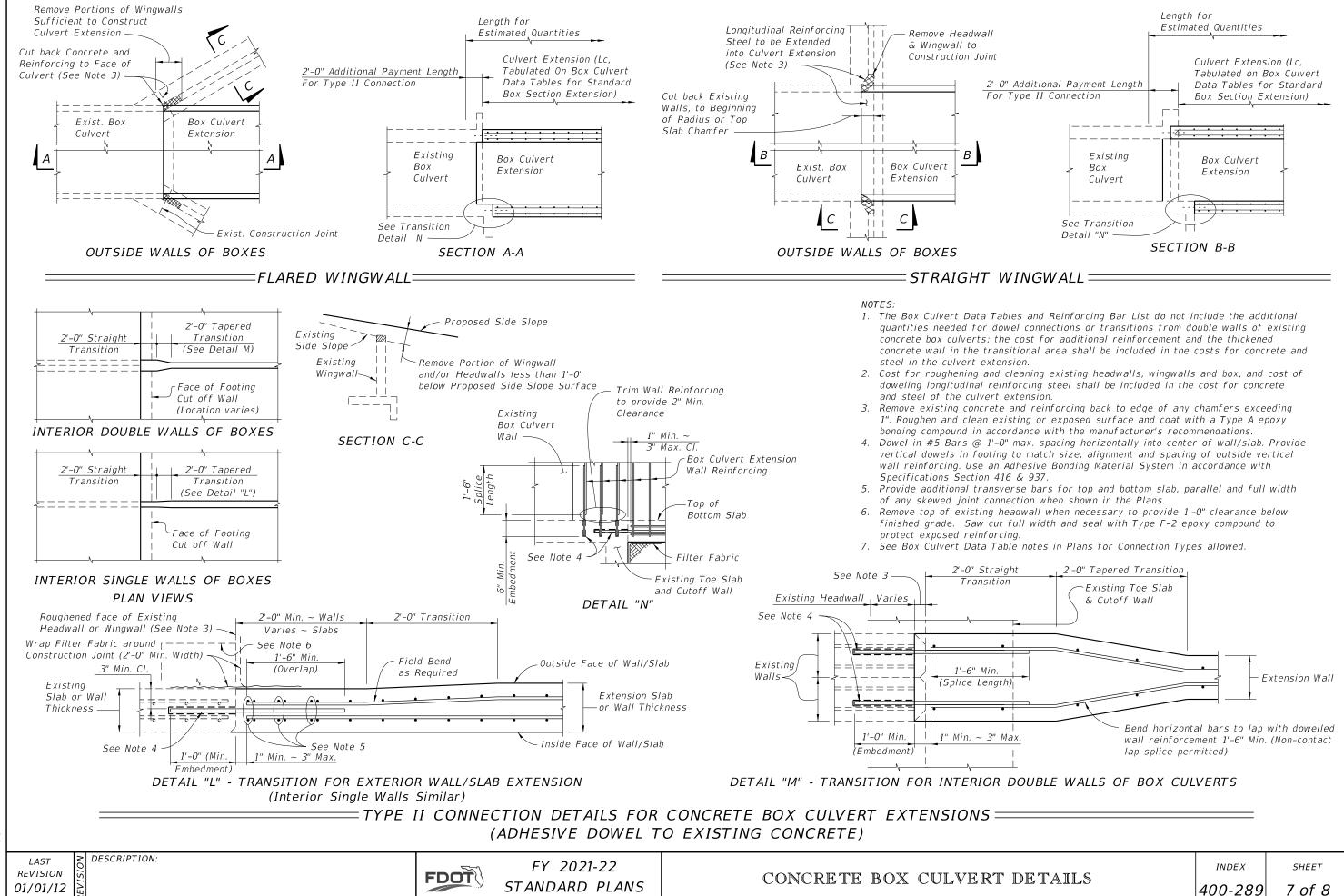
SHEET 2 of 8

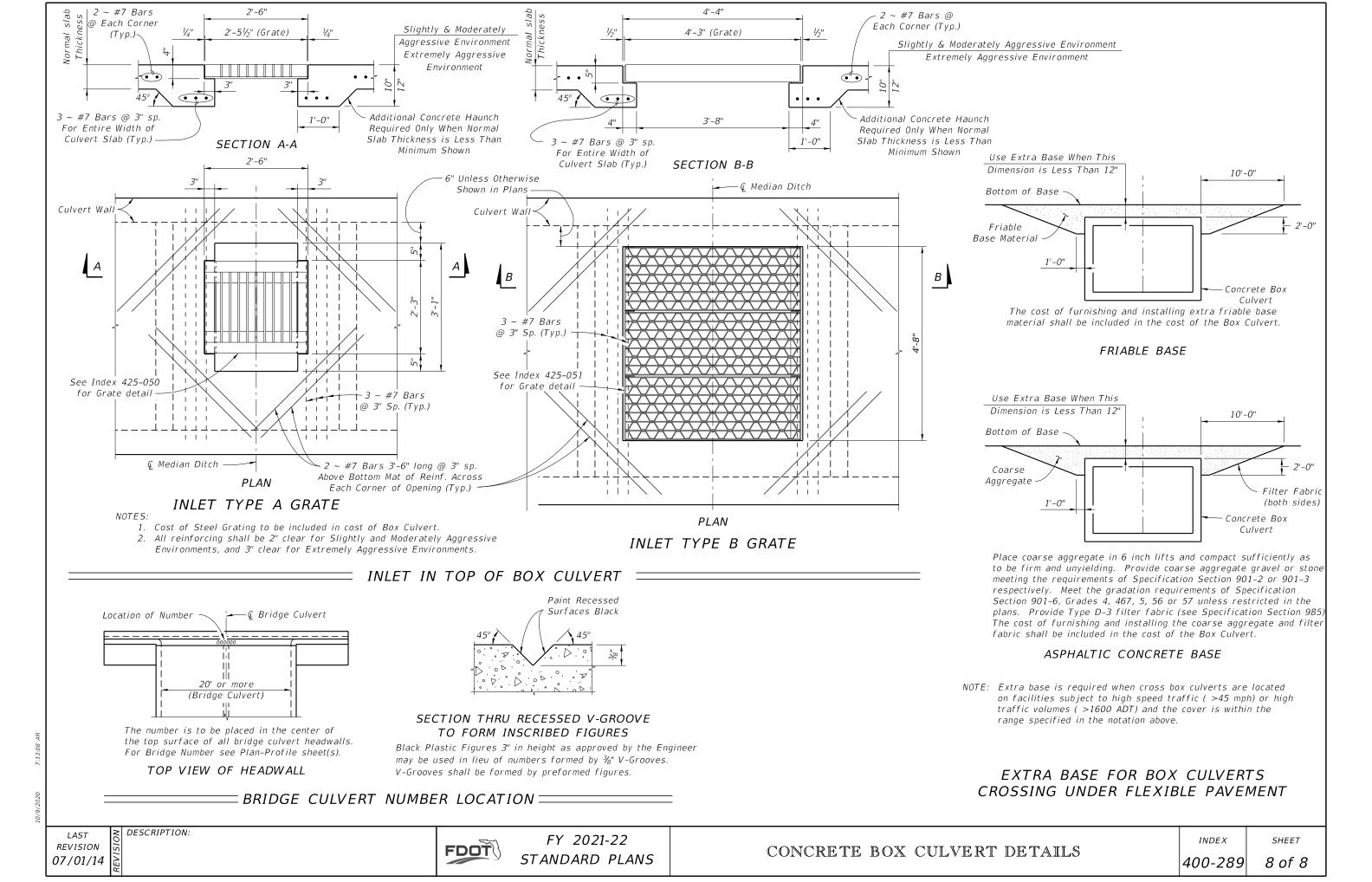


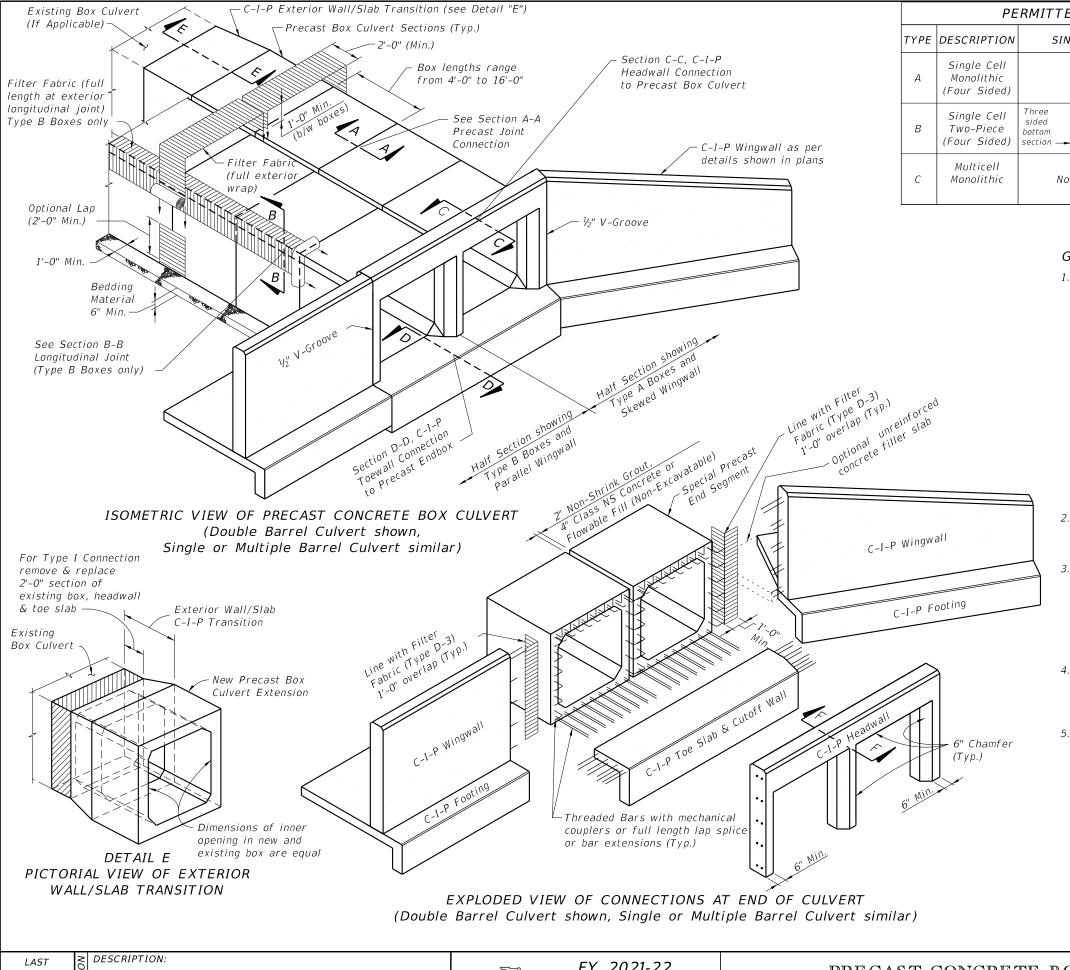












PERMITTED PRECAST ALTERNATE BOX SECTIONS SINGLE BARREL MULTIPLE BARRELS DESIGN NOTES Index 400-292 Contractor Design section Contractor Design Contractor Design Not Applicable

GENERAL NOTES:

1. Specifications:

FDOT Standard Specifications for Road and Bridge Construction, Section 410 (current edition, and supplements thereto).

Concrete (Precast):

Class III or Class II Modified (5,000 psi) for slightly aggressive environments.

Class IV (5,500 psi) for moderately to extremely aggressive environments.

Concrete (Cast-In-Place):

Class II (3,400 psi) for slightly aggressive environments. Class IV (5,500 psi) for moderately to extremely aggressive environments.

Reinforcing Steel:

Maintain minimum clearance of 2" for slightly and moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. Equal area substitution of welded wire (WWR) reinforcement is permitted.

- 2. Work this Index with the Cast-In-Place Concrete Box Culvert Details and Data Tables shown in the plans, Index 400-289 and the Precast Concrete Box Culverts shown in the shop drawings.
- 3. All joints between precast sections must be tongue & groove with joint sealant. Joints between cast-in-place & precast sections shall have longitudinal reinforcing extending from top, bottom & both side slabs of the precast box tied to the cast-in-place reinforcement. Single barrel culverts may have precast headwalls cast integrally with the end segment when approved by the Engineer.
- 4. Extension of existing multiple barrel box culverts with multiple single cell precast box culverts is not permitted unless approved by the District Structures Engineer. Full transition details must be shown in the shop drawings when approved.
- 5. Culverts larger than the specified size may be substituted with no additional payment to the Contractor. Substitution must be approved by the Engineer, minimum earth cover and invert elevations shown in the Contract Documents must be maintained.

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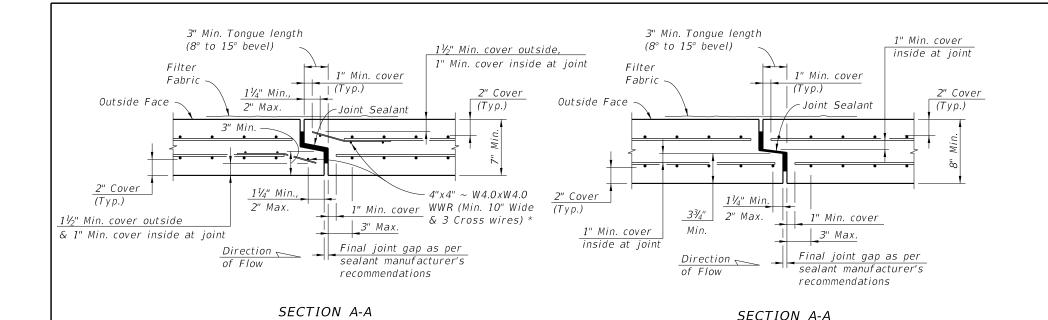
FY 2021-22 STANDARD PLANS

PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

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400-291



(2" Cover - Thin Wall Detail)

reinforcing utilizes WWR, extend wall and slab

reinforcing into the joint and bend to maintain cover in lieu of 4"x4" ~ W4.0xW4.0 WWR at joint.

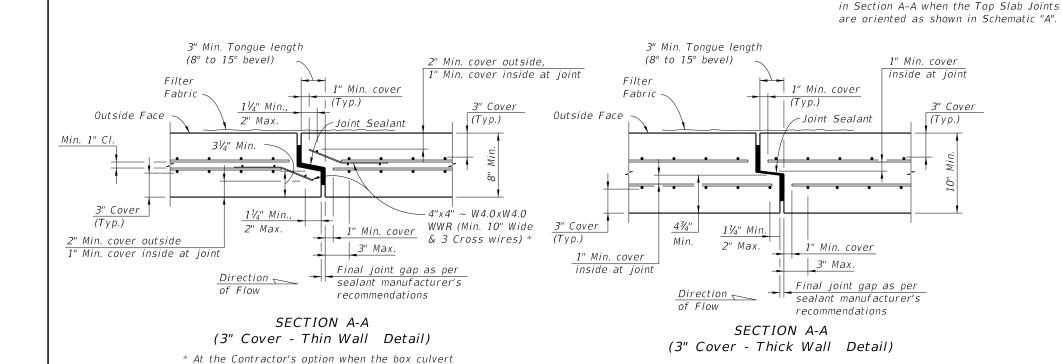
of box to allow bending of the WWR.

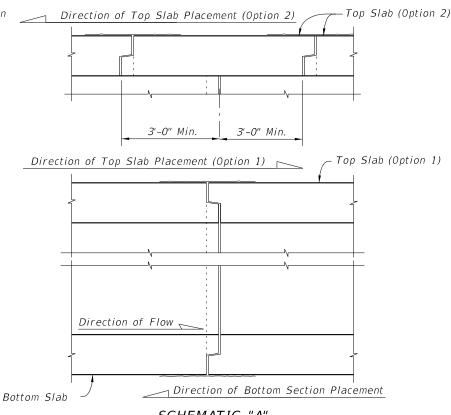
Transverse wire in tongue may be cut at corners

See Section A-A 3" Min. Tongue length for reinforcing (8° to 15° bevel) cover requirements 3" Min. 3" Min. 3" Min. -Joint Sealant Provide WWR or extend reinforcing into tongue (See Section A-A)

ALTERNATE BOTTOM SLAB TRANSVERSE JOINT TYPICAL SECTION (DOUBLE-SIDED TONGUE & GROOVE JOINT)

(All reinforcing not shown for clarity)





SCHEMATIC "A" TYPE B BOX SECTION PLACEMENT FOR SINGLE TONGUE & GROOVE JOINTS

= TWO-PIECE PRECAST SEGMENT ADDITIONAL JOINT DETAILS (TYPE B BOX)

PRECAST SEGMENT TO SEGMENT TONGUE & GROOVE TRANSVERSE JOINTS =

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(2" Cover - Thick Wall Detail)

NOTE:

Bottom Slab Joints in Type B Boxes may

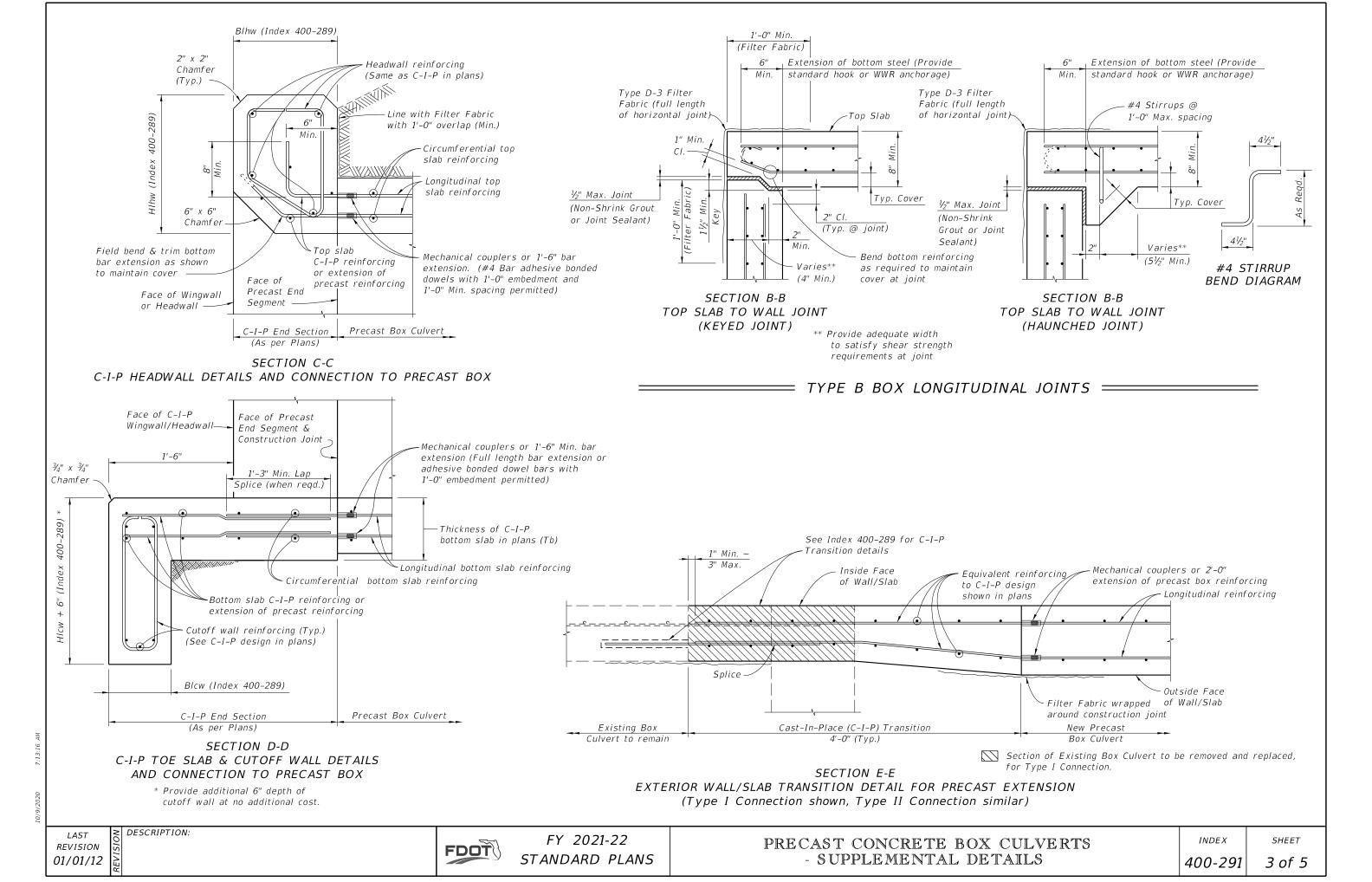
be single tongue & groove joints as shown

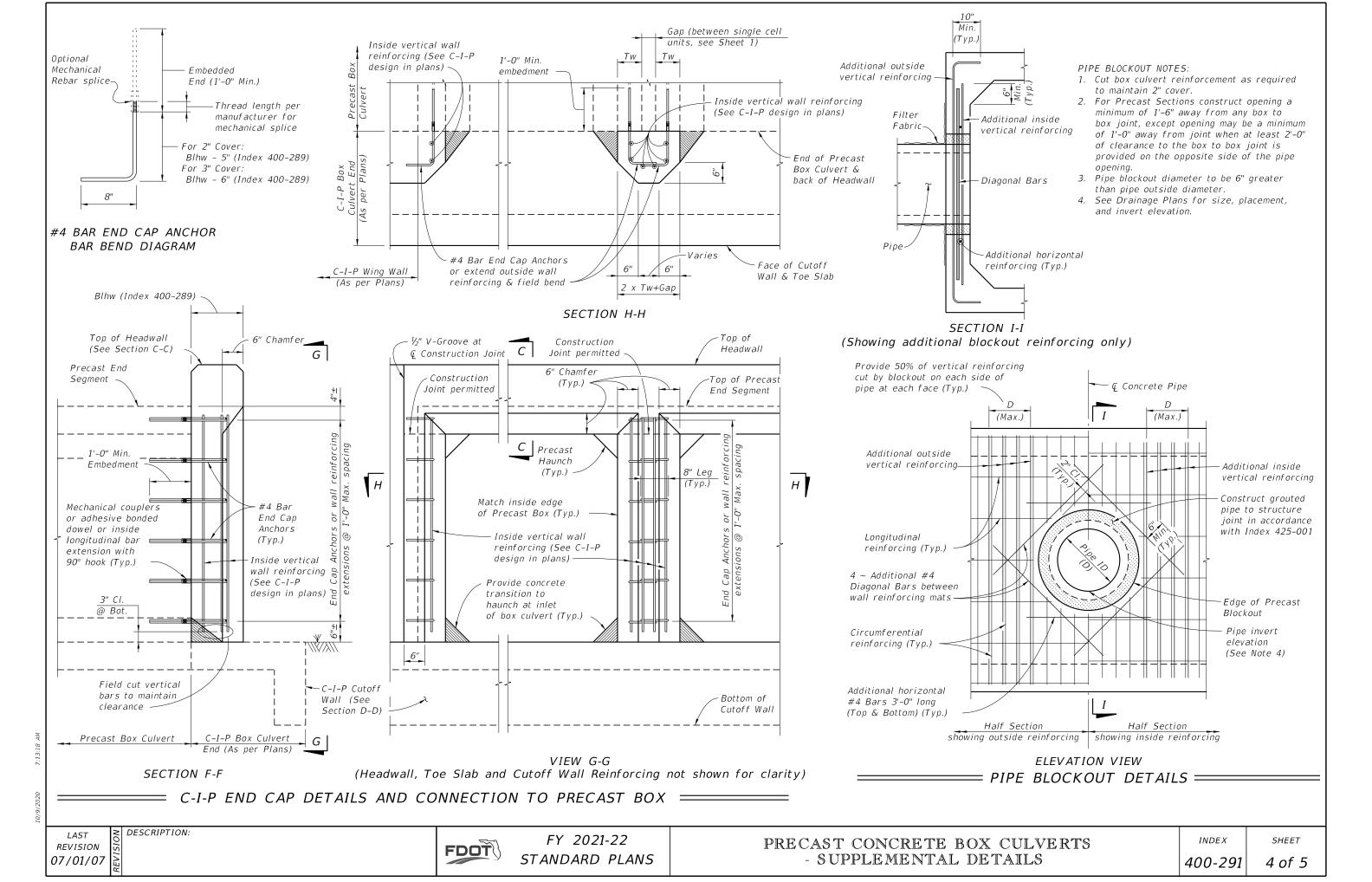
PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

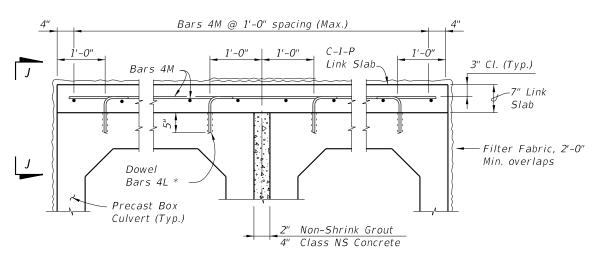
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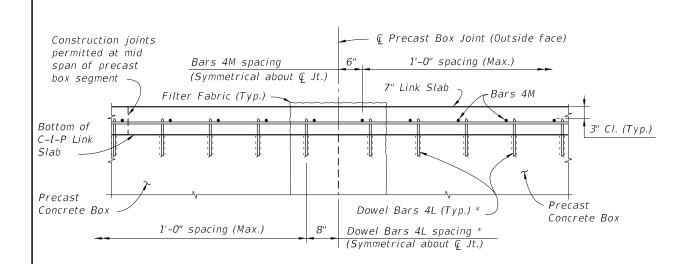






LINK SLAB TYPICAL SECTION (Multiple Barrel Culvert shown, Single Barrel Culvert similar)

* Install dowels with an Adhesive Bonding Material System in accordance with Specification Section 416. The Contractor may substitute mechanical couplers in lieu of adhesive bonded dowels. Shift dowels to clear box culvert reinforcing.



VIEW J-J

LINK SLAB NOTES:

1. Provide a Cast-In-Place Link Slab to ensure uniform joint opening of precast box culverts when the differential settlement shown in the plans exceeds the following limits, except that a Link Slab is not required for differential settlements less than 1/2".

$$\Delta Y \leq \frac{(L)^2}{760 \times R \times W}$$

Where:

 $\Delta Y = Maximum Long-Term Differential Settlement (ft.)$

R = Exterior height of Box Culvert (ft.)

W = Length of Box Culvert Segments (ft.)

L = Effective length for single curvature deflection (ft.)

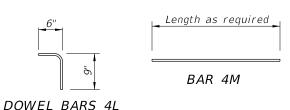
2. Extend Link Slab to back face of headwalls and to limits of existing box culverts for extensions.

ESTIMATED LINK SLA	B QUANTI	TIES
ITEM	UNIT	QUANTITY
Class II or IV Concrete (Culvert)	CY/SF	0.0216
Reinforcing Steel (Roadway)	Lb./SF	1.52
		·

NOTE: Estimated quantities are based the plan area of precast box slabs, and are provided for information only. No additional payment will be made for Link Slabs where these are required for the precast box culverts.

	BILL OF RE	INFORCING STEE	<u>L</u>
MARK	SIZE	NO. REQ'D	LENGTH
L	4	2 per Barrel/Ft.	1'-3"
М	4	As Reqd.	As Reqd.

REINFORCING STEEL BENDING DIAGRAMS

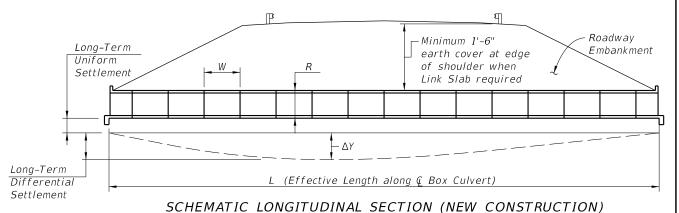


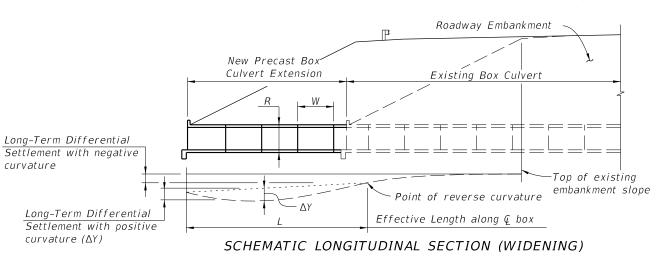
NOTES:

- 1. All bar dimensions are out to out.
- 2. Lap splice length for Bars 4M is 1'-4" minimum.

DESIGN NOTE:

1. Link Slab required when joint openings from differential settlement exceed 1/8" as determined in Link Slab Note 1.





= DIFFERENTIAL SETTLEMENT COUNTERMEASURES FOR PRECAST BOX CULVERTS =

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DESCRIPTION:

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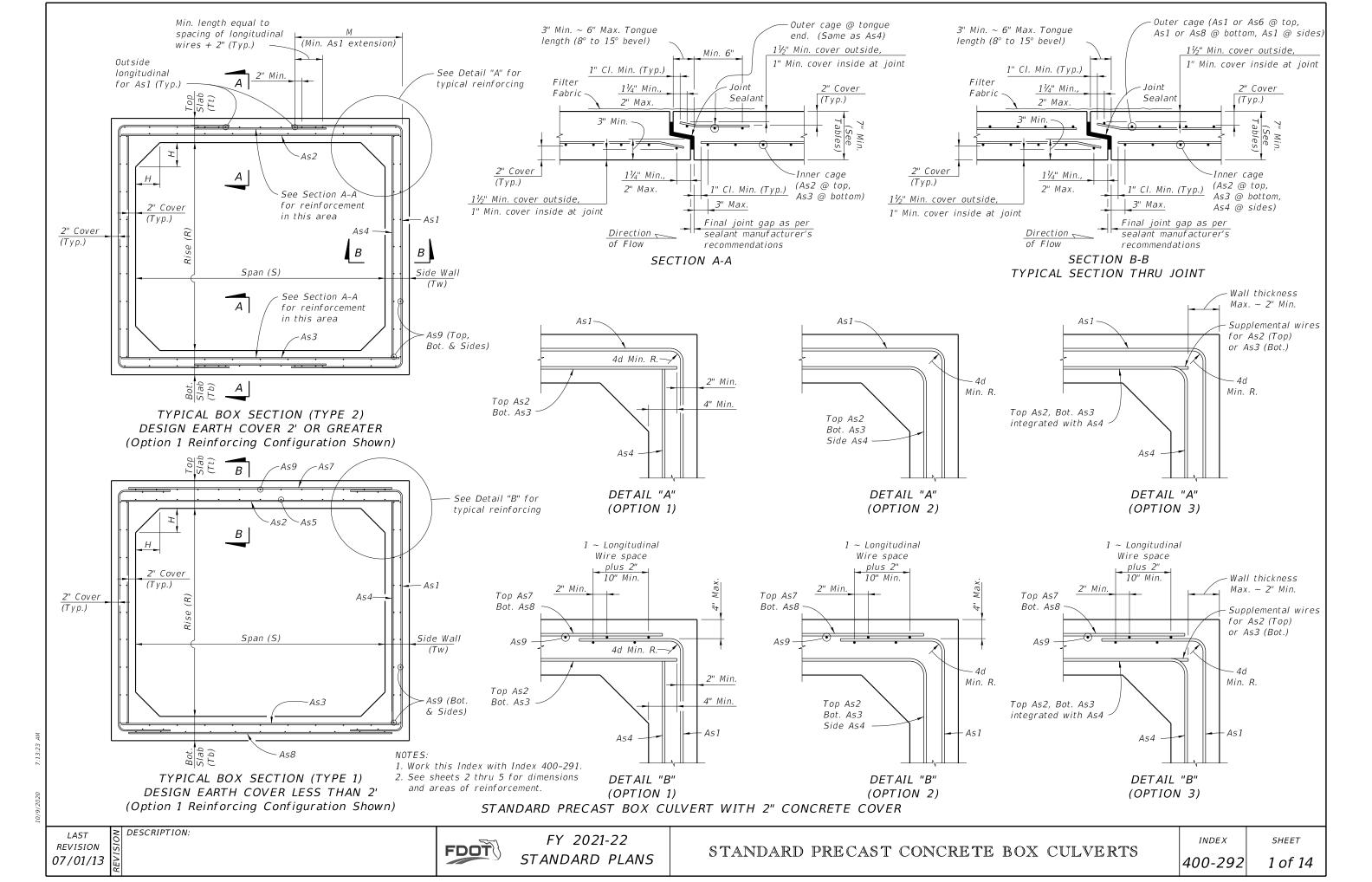
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PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

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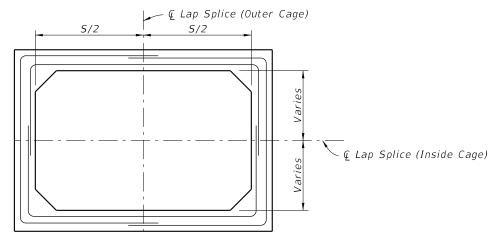


GENERAL NOTES:

- 1. These precast designs may be substituted for cast-in-place box culverts designed to AASHTO LRFD Bridge Design Specifications, 4th Edition. Designs are based on the design criteria shown in FDOT Structures Design Guidelines.
- 2. Loading: HL-93 & any fill heights between the minimum & maximum shown.
- 3. Only one design of precast box culvert is to be used for any installation.
- 4. Reinforcing steel must consist of smooth or deformed welded wire reinforcement (WWR) meeting the requirements of Specification Section 931. Longitudinal reinforcement may consist of reinforcing bars meeting the requirements of Specification Section 931. Minimum cover must be 2" for slightly or moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. The spacing of circumferential wires must not be less than 2" nor more than 4". The spacing of longitudinal wires or bars must not be more than 8".
- 5. As9 longitudinal wires must have a minimum cross-sectional area of 40% of the circumferential wires, but not less than a W2.5 or D4.0 for WWR, or #3 bars for deformed bars.
- 6. Welding of reinforcement must be limited to the locations shown in ASTM C1577 and in accordance with ANSI/AWS D1.4 "Structural Welding Code Reinforcing Steel".
- 7. For alternate reinforcing configuration Options 2 and 3 shown in Detail "A" and "B" (Sheet 1), As1 may be extended to the middle of either slab and lap spliced with As7 and As8. As4 may be lap spliced at any location or connected to As2 or As3 at corners by welding.
- 8. Haunch dimensions may vary between the minimum and maximum dimensions shown in the Design Tables but only one haunch dimension must be used within the full length of the box culvert installation.

TABLE	1A - S	STANI	DARD	PRECA	ST BOX CU	LVERT	T DES	SIGNS	(2" (COVE	R) - 3	′&	4' SP.	ANS
SPAN x RISE		/ WAL			DESIGN			R	EINFOR			5		As1 EXT.
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M) (in.)
(FL.)	(in.)	(in.)	(in.)	(in.)	TUP SLAD	As 1	As2	As3	As4	As5	As7	As8	A59	(111.)
					0.33' - <2'	0.17	0.29	0.21	0.17	0.17	0.17	0.17		
				4	2' - <3'	0.13	0.28	0.21	0.09	-	-	-		31
					3' - <5'	0.09	0.17	0.17	0.09	-	-	-		31
					5' - 10'	0.09	0.17	0.17	0.09	-	-	-		31
3' x 3'	7	7	7	to	15'	0.09	0.17	0.17	0.09	-	-	-		31
					20'	0.12	0.17	0.17	0.09	-	-	-		31
					25'	0.14	0.18	0.18	0.09	-	-	-		31
				8	30'	0.17	0.21	0.22	0.09	-	-	-		31
					35'	0.19	0.25	0.25	0.09	-	-	-	, ,	31
					0.33' - <2'	0.19	0.38	0.26	0.17	0.19	0.17	0.19	e 5	-
				4	2' - <3'	0.19	0.38	0.26	0.09	-	-	-	Note	38
				4	3' - <5'	0.14	0.20	0.22	0.09	-	-	-	<u> </u>	38
4' x 3'	7	7	7	to	5' - 10'	0.11	0.17	0.17	0.09	-	-	-	General	38
4 1 3	/	′	′	10	15'	0.15	0.17	0.18	0.09	-	-	-	епе	38
				8	20'	0.20	0.23	0.23	0.09	-	-	-		38
				0	25'	0.24	0.28	0.29	0.09	-	-	-	See	38
					30'	0.29	0.34	0.35	0.09	-	-	-	, ,	38
					0.33' - <2'	0.19	0.41	0.28	0.17	0.21	0.17	0.19		-
					2' - <3'	0.19	0.41	0.28	0.09	-	-	-		38
				4	3' - <5'	0.14	0.21	0.24	0.09	-	-	-		38
4' x 4'	7	7	7	t .	5' - 10'	0.12	0.17	0.17	0.09	-	-	-		38
	/	′	′	to	15'	0.16	0.19	0.20	0.09	-	-	-		38
				8	20'	0.21	0.25	0.25	0.09	-	-	-		38
				Ö	25'	0.26	0.31	0.32	0.09	-	-	-		38
					30'	0.31	0.37	0.38	0.09	-	-	-		38

- 9. Submittal of redesign calculations are not required for any increase to the slab and/or wall thickness when the minimum reinforcement areas shown in the Design Tables are provided.
- 10. For Design Earth Cover greater than 10 feet, the Contractor may interpolate the required areas of reinforcement and slab or wall thickness. Interpolated areas of reinforcement, slab or wall thickness must be approved by the Engineer.
- 11. Minimum length of precast box segments is 4 feet and maximum length is 16 feet.
- 12. See Index 400-291 for connections to wingwalls, headwalls and other general details.



SCHEMATIC OF LAP SPLICE LOCATIONS FOR OPTION 2 & 3 REINFORCING CONFIGURATIONS

TABL	.E 1B	- STA	ANDAI	RD PRE	CAST BOX	CULVE	ERT D	ESIG	NS (2	" COV	/ER) -	- 3'	& 4'.	SPANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.20	0.26	0.32	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.16	0.25	0.31	0.10	-	-	-		31
					3' - <5'	0.10	0.20	0.20	0.10	-	-	-		31
					5' - 10'	0.10	0.20	0.20	0.10	-	-	-		31
3' x 3'	8	8	8	to	15'	0.10	0.20	0.20	0.10	-	-	-		31
					20'	0.10	0.20	0.20	0.10	-	-	-		31
					25'	0.11	0.20	0.20	0.10	-	-	-		31
				8	30'	0.13	0.20	0.20	0.10	-	-	-		31
					35'	0.15	0.21	0.21	0.10	-	-	-	1	31
					0.33' - <2'	0.20	0.31	0.22	0.20	0.20	0.20	0.20	5 5	-
				4	2' - <3'	0.12	0.31	0.22	0.10	-	-	-	Note	38
				4	3' - <5'	0.12	0.20	0.20	0.10	-	-	-	\ <u>\</u>	38
4' x 3'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	General	38
4 ^ 3	0	0	0	"	15'	0.12	0.20	0.20	0.10	-	-	-	епе	38
				8	20'	0.16	0.20	0.20	0.10	-	-	-		38
					25'	0.19	0.24	0.24	0.10	-	-	-	See	38
					30'	0.22	0.28	0.29	0.10	-	-	-] ,	38
					0.33' - <2'	0.20	0.33	0.24	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.17	0.33	0.24	0.10	-	-	-		38
				4	3' - <5'	0.12	0.20	0.20	0.10	-	-	-		38
4' x 4'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-		38
4 ^ 4	0	U		"	15'	0.13	0.20	0.20	0.10	-	-	-		38
				8	20'	0.16	0.21	0.22	0.10	-	-	-		38
					25'	0.20	0.26	0.27	0.10	-	-	-		38
					30'	0.23	0.31	0.32	0.10	-	-	-		38

NOTES: 1. See Sheet 1 for Reinforcing Details and dimension locations.

2. See Sheet 14 for WWR Bending Diagram.

DESCRIPTION:

TABL	LE 2A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT D	DESIG	NS (2	?" COI	/ER)	- 5'	& 6'	SPANS
PAN x RISE		/ WAL	L THIC	KNESS	DESIGN			R		RCEMEN		ıS		As1 EX
S) (R)	TOP	BOT.	l	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGT
(F+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	A58	A59	(in.)
					0.33' - <2'	0.31	0.48	0.42	0.17	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.48	0.42	0.09	-	_	_		45
				,	3' - <5'	0.20	0.27	0.27	0.09	_	_	_		36
5' x 3'	7	7	7	+0	5' - 10'	0.17	0.19	0.21	0.09	_	_	_		36
<i>3</i>	′	′	′	to	15'	0.24	0.25	0.25	0.09	_	_	_		35
					20'	0.32	0.23	0.23	0.09	_				35
				8	25'					_		_		
						0.39	0.41	0.42	0.09	-	-	-		35
					30'	0.47	0.50	0.50	0.09	-	-	-		35
					0.33' - <2'	0.30	0.51	0.45	0.17	0.23	0.21	0.30		
				4	2' - <3'	0.30	0.51	0.45	0.09	-	ı	1		45
					3' - <5'	0.18	0.30	0.29	0.09	-	-	-		45
5' x 4'	7	7	7	to	5' - 10'	0.17	0.21	0.23	0.09	_	-	-		36
					15'	0.24	0.27	0.28	0.09	-	-	_		35
				8	20'	0.31	0.36	0.37	0.09	_	_	_		35
					25'	0.39	0.45	0.46	0.09	_	_	_		35
					30'	0.46	0.55	0.56	0.09	_	_	_		35
					0.33' - <2'	0.30	0.53	0.48	0.17	0.24	0.21	0.30		-
				4	2' - <3'	0.29	0.53	0.48	0.09	-	-	-		45
					3' - <5'	0.19	0.31	0.31	0.09	-	-	_		45
5' x 5'	7	7	7	to	5' - 10'	0.19	0.22	0.25	0.09	-	-	-		45
					15'	0.26	0.29	0.31	0.09	_	-	-		36
				8	20'	0.34	0.39	0.40	0.09	-	_	_		35
					25'	0.41	0.49	0.50	0.09	-	_	_		35
					30'	0.49	0.59	0.61	0.09	_	_	_		35
	7.5	7	7				0.54			0.22	0.25	0.20	10	
	7.5	/	/		0.33' - <2'	0.39		0.48	0.17			0.39	7	-
				4	2' - <3'	0.39	0.58	0.49	0.09	-	-	-	Note	43
					3' - <5'	0.28	0.36	0.36	0.09	-	-	-	>	39
6' x 3'	7	7	7	to	5' - 10'	0.25	0.26	0.28	0.09	-	-	-	General	39
					15'	0.36	0.34	0.34	0.09	_	_	_	lei	38
				12	20'	0.47	0.46	0.46	0.09	-	-	_	3e	38
	7	7.5	7		25'	0.59	0.57	0.55	0.09	_	_	_		38
	8	8	7		30'	0.60	0.64	0.64	0.09	_	_	_	See	38
	7.5	7	7		0.33' - <2'	0.37	0.58	0.52	0.17	0.24	0.23	0.37		- 30
	7.5													12
				4	2' - <3'	0.37	0.61	0.53	0.09	-	-	1-1		43
					3' - <5'	0.26	0.39	0.39	0.09	-	-	-		39
6' x 4'	7	7	7	to	5' - 10'	0.24	0.28	0.31	0.09	-	-	-		39
					15'	0.35	0.37	0.38	0.09	-	-	-		38
		L		12	20'	0.46	0.50	0.50	0.09	-	-	_		38
	7	7.5	7		25'	0.56	0.63	0.60	0.09	-	-	_		38
	8	8	7	1	30'	0.58	0.69	0.69	0.09	-	_	_		38
	7.5	7	7		0.33' - <2'	0.36	0.60	0.56	0.17	0.25	0.22	0.36		-
	1.5	- ' -	 '		2' - <3'	0.36	0.64	0.56	0.17	-	-	-		43
				4										
CI =:	_	_	_		3' - <5'	0.26	0.410	0.42	0.09	-	-	_		43
6' x 5'	7	7	7	to	5' - 10'	0.25	0.30	0.33	0.09	-	-	_		39
					15'	0.34	0.40	0.41	0.09	-	-	-		38
				12	20'	0.46	0.54	0.54	0.09	-	-	-		38
	7	7.5	7		25'	0.56	0.67	0.65	0.09	-	-	-		38
	8	8	8]	30'	0.60	0.74	0.74	0.09	_	-	_		38
	7.5	7	7		0.33' - <2'	0.36	0.63	0.59	0.17	0.26	0.22	.036		_
	1.5				2' - <3'		0.67	0.59	0.17	-	-			52
				4		0.35								
-	_	_	_		3' - <5'	0.27	0.43	0.44	0.09	-	-	_		52
6' x 6'	7	7	7	to	5' - 10'	0.27	0.32	0.35	0.09	-	-	-		43
					15'	0.38	0.43	0.44	0.09	-	-	-		39
				12	20'	0.50	0.57	0.59	0.09	-	-	-		39
	7	7.5	7	1	25'	0.60	0.72	0.70	0.09	-	-	-		38
	8	8	7	1	30'	0.67	0.78	0.79	0.09	_	_	_		38
				1		0.07	1 0., 0	L J., J	1 0.00				1	1 50

TABL	E 2B	- ST	ANDA	RD PRE	CAST BOX	CULVI	ERT D	DESIG	NS (2	?" CO	VER)	- 5'	& 6'	SPANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.26	0.39	0.36	0.20	0.20	0.20	0.26		_
				4	2' - <3'	0.26	0.39	0.36	0.10	-	-	-		45
					3' - <5'	0.16	0.23	0.24	0.10	-	-	-		36
5' x 3'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	-	-	-		36
					15'	0.19	0.21	0.22	0.10	-	-	-		35
				8	20'	0.24	0.28	0.28	0.10	-	-	-		35
					25'	0.30	0.34	0.35	0.10	-	-	-		35
					30'	0.36	0.41	0.41	0.10	-	-	-	-	35
					0.33' - <2'	0.25	0.42	0.38	0.20	0.20	0.20	0.25	-	-
				4	2' - <3'	0.25	0.42	0.38	0.10	-	-	-	-	45
5' x 4'	8	8	8		3' - <5' 5' - 10'	0.16	0.25	0.25	0.10	-	-	-	-	45 36
3 X 4	0	0	"	to	3 - 10 15'	0.13	0.23	0.20	0.10	_	_	_	-	35
					20'	0.19	0.23	0.24	0.10	_	_	_	1	35
				8	25'	0.24	0.37	0.31	0.10	_	_	_	-	35
					30'	0.35	0.45	0.46	0.10	_	_	_	-	35
			-		0.33' - <2'	0.25	0.43	0.41	0.20	0.20	0.20	0.25	1	_
				4	2' - <3'	0.25	0.44	0.41	0.20	-	-	-	1	45
				4	3' - <5'	0.16	0.26	0.27	0.10	_	_	_	-	45
5' x 5'	8	8	8	to	5' - 10'	0.15	0.20	0.22	0.10	_	_	_	1	45
J . J	Ü			"	15'	0.20	0.25	0.26	0.10	_	_	_	1	36
				8	20'	0.26	0.32	0.33	0.10	_	_	_	1	35
					25'	0.32	0.40	0.41	0.10	-	-	_		35
					30'	0.37	0.48	0.49	0.10	-	-	-	1	35
					0.33' - <2'	0.32	0.47	0.41	0.20	0.20	0.25	0.32	72	-
				4	2' - <3'	0.32	0.47	0.41	0.10	-	-	-	Note	43
					3' - <5'	0.23	0.30	0.31	0.10	-	-	-		39
6' x 3'	8	8	8	to	5' - 10'	0.19	0.22	0.24	0.10	-	-	-	General	39
					15'	0.28	0.29	0.29	0.10	-	-	-	ine.	38
				12	20'	0.36	0.38	0.38	0.10	-	-	-		38
					25'	0.45	0.47	0.47	0.10	-	-	-	See	38
					30'	0.54	0.57	0.57	0.10	-	-	-	S	38
					0.33' - <2'	0.31	0.50	0.44	0.20	0.21	0.23	0.31	_	_
				4	2' - <3'	0.31	0.50	0.44	0.10	-	-	-	-	43
CI 41	0	0			3' - <5'	0.23	0.32	0.34	0.10	-	_	-	-	39
6' x 4'	8	8	8	to	5' - 10'	0.19	0.24	0.26	0.10	-	-	-	-	39
				1.2	15' 20'	0.27	0.31	0.32	0.10	-	-	-	-	38 38
				12	20 25'	0.33	0.41	0.41	0.10	-	_	-	-	38
					30'	0.43	0.62	0.51	0.10	_	_	_	-	38
					0.33' - <2'	0.30	0.52	0.02	0.20	0.22	0.22	0.30	1	-
				4	2' - <3'	0.30	0.52	0.47	0.20	-	-	-	1	43
				"	3' - <5'	0.22	0.34	0.36	0.10	_	_	_	1	43
6' x 5'	8	8	8	to	5' - 10'	0.20	0.26	0.28	0.10	-	-	_		39
		-	-		15'	0.27	0.33	0.34	0.10	-	_	_	1	38
				12	20'	0.36	0.44	0.45	0.10	-	-	-	1	38
					25'	0.44	0.55	0.55	0.10	-	-	-	1	38
					30'	0.52	0.66	0.67	0.10	-	-	-]	38
					0.33' - <2'	0.30	0.54	0.50	0.20	0.22	0.22	0.30		-
				4	2' - <3'	0.30	0.54	0.50	0.10	-	-	_		52
					3' - <5'	0.23	0.36	0.38	0.10	-	-	-		52
6' x 6'	8	8	8	to	5' - 10'	0.21	0.27	0.30	0.10	-	-	-		43
					15'	0.29	0.35	0.37	0.10	-	-	-		39
				12	20'	0.38	0.47	0.48	0.10	-	-	-		39
					25'	0.47	0.59	0.60	0.10	-	-	-		38
I			1	I	30'	0.55	0.70	0.71	0.10	_	_	_	1	38

LAST REVISION 07/01/13

≥ DESCRIPTION:

FDOT

T	ABLE	3 - 5	STANI	DARD P	RECAST BO	X CU	LVERT	DES	<i>IGNS</i>	(2" (COVER	R) - 7	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.37	0.58	0.49	0.20	0.22	0.29	0.37		-
				4	2' - <3'	0.37	0.58	0.49	0.10	-	-	-		43
				t o	3' - <5'	0.30	0.40	0.42	0.10	-	-	-		43
7' x 4'	8	8	8	to	5' - 10'	0.26	0.30	0.33	0.10	-	-	-		43
				12	15'	0.37	0.40	0.40	0.10	-	-	-		41
				12	20'	0.49	0.53	0.53	0.10	-	-	-		41
	8	8	8	7 to	25'	0.60	0.67	0.66	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.68	0.79	0.78	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.60	0.53	0.20	0.23	0.28	0.36		-
				4	2' - <3'	0.36	0.60	0.53	0.10	-	-	-		47
				to	3' - <5'	0.30	0.42	0.45	0.10	-	-	-		43
7' x 5'	8	8	8	to	5' - 10'	0.26	0.32	0.35	0.10	-	-	-		43
				12	15'	0.37	0.43	0.44	0.10	-	-	-	5	41
				12	20'	0.48	0.57	0.57	0.10	-	-	-	te	41
	8	8	8	7 to	25'	0.60	0.72	0.72	0.10	-	-	-	No	41
	8.5	8.5	8	12	30'	0.67	0.84	0.84	0.10	-	-	-	General Note	41
				4	0.33' - <2'	0.36	0.63	0.56	0.20	0.24	0.27	0.36	neı	-
				4	2' - <3'	0.36	0.63	0.56	0.10	-	-	-	е9	59
				to	3' - <5'	0.29	0.44	0.47	0.10	-	-	-	See	47
7' x 6'	8	8	8	10	5' - 10'	0.27	0.34	0.37	0.10	-	-	-	S	43
				12	15'	0.38	0.46	0.46	0.10	-	-	-		41
				12	20'	0.49	0.60	0.61	0.10	-	-	-		41
	8	8	8	7 to	25'	0.61	0.76	0.76	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.69	0.89	0.89	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.65	0.58	0.20	0.25	0.27	0.36		-
				4	2' - <3'	0.36	0.65	0.58	0.10	-	-	-		59
				t 0	3' - <5'	0.30	0.46	0.50	0.10	-	-	-		59
7' x 7'	8	8	8	to	5' - 10'	0.30	0.35	0.50	0.10	-	-	-		47
				12	15'	0.41	0.48	0.50	0.10	-	-	-		43
				12	20'	0.53	0.64	0.65	0.10	-	-	-		43
	8	8	8	7 to	25'	0.65	0.80	0.81	0.10	-	-	-		43
	8.5	9	8	12	30'	0.72	0.92	0.91	0.10	-	-	-		41

T	ABLE	4 - 5	STANI	DARD P	RECAST BO	X CU	LV ER 7	DES	IGNS	(2" (COVER	R) - 8	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R			T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			T	T		T	T		(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	A59	(111.)
	9	8.5	8	4	0.33' - <2'	0.40	0.60	0.52	0.20	0.22	0.28	0.39		-
					2' - <3'	0.45	0.66	0.54	0.10	-	-	-		50
	_			to	3' - <5'	0.39	0.48	0.50	0.10	-	-	-		50
8' x 4'	8	8	8		5' - 10'	0.34	0.38	0.40	0.10	-	-	-		45
				12	15'	0.49	0.51	0.50	0.10	-	-	-		41
					20'	0.65	0.68	0.66	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.76	0.83	0.80	0.10	_	_	-		41
	9.5	9.5	8	12	30'	0.79	0.94	0.92	0.10	-	-	-		41
	9	8.5	8	4	0.33' - <2'	0.38	0.65	0.59	0.20	0.22	0.30	0.37		-
					2' - <3'	0.43	0.69	0.58	0.10	-	-	-		50
0, 5,	0			to	3' - <5'	0.37	0.51	0.53	0.10	_	-	-		45
8' x 5'	8	8	8		5' - 10'	0.33	0.41	0.42	0.10	-	-	-		45
				12	15'	0.48	0.54	0.53	0.10	-	-	-		41
	0.5	0.5		0 ,	20'	0.63	0.73	0.70	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.74	0.88	0.86	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.77	1.00	0.98	0.10	-		- 0.71		41
	9	9	8	4	0.33' - <2'	0.32	0.65	0.58	0.20	0.23	0.25	0.31	e 5	-
					2' - <3'	0.42	0.71	0.61	0.10	-	-	-	Note	50
8' x 6'	8	8	8	to	3' - <5' 5' - 10'	0.37	0.54	0.56	0.10	-	-	-	< -	50 45
8	8	8	8			0.34	0.43	0.45	0.10	-	-	-	General	
				12	15' 20'	0.49	0.37	0.76	0.10	-	-	-	ene	41
	8.5	8.5	8	8 to	25'	0.74	0.77	0.70	0.10		_	_	9 6	41
•	9.5	9.5	8	12	30'	0.74	1.05	1.04	0.10		_	_	See	41
	9.5	9.5	8	12	0.33' - <2'	0.78	0.67	0.60	0.10	0.24	0.24	0.31	- '	-
	9	9	0	4	2' - <3'	0.31	0.74	0.64	0.20	-	0.24	0.51		55
					3' - <5'	0.42	0.74	0.59	0.10		_	_		55
8' x 7'	8	8	8	to	5' - 10'	0.36	0.45	0.33	0.10		_	_		50
	U				15'	0.51	0.43	0.47	0.10	_	_	_		45
				12	20'	0.66	0.81	0.80	0.10		_	_		41
	8.5	8.5	8	8 to	25'	0.78	0.98	0.97	0.10	_	_	_		41
}	9.5	9.5	8	12	30'	0.84	1.10	1.09	0.10	_	_	_		41
	9	9	8		0.33' - <2'	0.32	0.68	0.62	0.20	0.24	0.25	0.32		
				4	2' - <3'	0.32	0.76	0.67	0.20	-	0.23	-		65
					3' - <5'	0.38	0.58	0.61	0.14	_	_	_		65
8' x 8'	8	8	8	to	5' - 10'	0.39	0.46	0.50	0.13	_	_	_		55
					15'	0.55	0.64	0.65	0.10	_	_	_		45
				12	20'	0.71	0.86	0.85	0.10	_	_	_		45
	8.5	8.5	8	8 to	25'	0.84	1.03	1.02	0.10	_	_	_		41
	9.5	9.5	8	12	30'	0.93	1.15	1.15	0.10	_	_	-		41
				1	l .						1			1

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- See Sheet 2 for General Notes.
 See Sheet 14 for Welded Wire Reinforcement Bending Diagram.



TABL	E 5 -	STA	NDAR	D PREC	CAST BOX C	ULVE	RT DI	ESIGN	IS (2"	COV	ER) -	9' SF	PANS	
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
	9.5	9.5	9	4	0.33' - <2'	0.41	0.62	0.53	0.22	0.23	0.34	0.38		-
				4	2' - <3'	0.44	0.65	0.54	0.11	-	-	-		54
				to	3' - <5'	0.39	0.53	0.51	0.11	-	-	_		49
9' x 5'	9	9	9	10	5' - 10'	0.35	0.42	0.44	0.11	-	-	-		49
				12	15'	0.50	0.56	0.55	0.11	-	-	-		44
				12	20'	0.65	0.75	0.73	0.11	-	-	_		44
	9.5	9.5	9	8 to	25'	0.77	0.92	0.90	0.11	-	-	-		44
	10.5	11	9	12	30'	0.81	1.05	1.02	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.38	0.64	0.56	0.23	0.23	0.33	0.37		-
[2' - <3'	0.43	0.67	0.57	0.11	-	-	-		54
				to	3' - <5'	0.37	0.55	0.54	0.11	-	-	_		49
9' x 6'	9	9	9	10	5' - 10'	0.35	0.45	0.47	0.11	-	-	-		49
				12	15'	0.49	0.60	0.59	0.11	-	-	-		44
				12	20'	0.65	0.80	0.78	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.76	0.98	0.95	0.11	-	-	_		44
	10.5	11	9	12	30'	0.80	1.10	1.08	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.37	0.67	0.59	0.22	0.23	0.32	0.37	5	-
				4	2' - <3'	0.42	0.69	0.60	0.11	-	-	-	te	59
				4-	3' - <5'	0.37	0.58	0.56	0.11	-	-	-	Note	54
9' x 7'	9	9	9	to	5' - 10'	0.36	0.47	0.49	0.11	-	-	-		49
				1.2	15'	0.50	0.63	0.63	0.11	-	-	-	ıer	44
				12	20'	0.66	0.84	0.80	0.11	-	-	-	General	44
	9.5	9.5	9	8 to	25'	0.77	1.02	1.00	0.11	-	-	-	See	44
	10.5	11	9	12	30'	0.81	1.15	1.13	0.11	-	-	-	Se	44
	9.5	9.5	9	4	0.33' - <2'	0.37	0.68	0.61	0.22	0.23	0.31	0.37		_
				4	2' - <3'	0.42	0.71	0.62	0.11	-	-	_		59
					3' - <5'	0.37	0.60	0.59	0.11	-	-	_		59
9' x 8'	9	9	9	to	5' - 10'	0.38	0.49	0.51	0.11	-	_	_		54
				12	15'	0.53	0.66	0.66	0.11	-	_	_		44
				12	20'	0.68	0.88	0.87	0.11	-	-	-		44
	9.5	9.5	9	8 to	25'	0.81	1.07	1.05	0.11	-	_	-		44
	10.5	11	9	12	30'	0.86	1.20	1.18	0.11	-	_	_		44
	9.5	9.5	9	4	0.33' - <2'	0.38	0.70	0.63	0.22	0.23	0.32	0.38		_
				4	2' - <3'	0.43	0.73	0.65	0.15	-	-	-		72
					3' - <5'	0.38	0.62	0.61	0.15	-	_	_		72
9' x 9'	9	9	9	to	5' - 10'	0.41	0.50	0.53	0.14	-	-	-		59
				1.3	15'	0.57	0.69	0.70	0.12	-	-	-		49
				12	20'	0.73	0.92	0.91	0.11	-	-	-		49
	9.5	10	9	8 to	25'	0.83	1.11	1.09	0.11	-	_	-		44
	10.5	11	9	12	30'	0.93	1.25	1.23	0.11	_	_	_		44
										l	L		<u> </u>	

TABL	E 6 -	STAI	VDARI	O PREC	AST BOX C	ULVE	RT DE	SIGN	5 (2"	COVI	ER) -	10' SI	PANS	
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.46	0.62	0.52	0.24	0.24	0.41	0.45		-
				4	2' - <3'	0.46	0.62	0.52	0.12	_	_	-		58
					3' - <5'	0.42	0.54	0.50	0.12	_	-	-		53
10' x 5'	10	10	10	to	5' - 10'	0.38	0.46	0.49	0.12	-	-	-		52
					15'	0.52	0.59	0.58	0.12	_	_	_		47
				12	20'	0.69	0.78	0.76	0.12	_	_	_		47
	10.5	10.5	10	8 to	25'	0.81	0.97	0.93	0.12	-	-	-		47
	11.5	12	10	12	30'	0.87	1.11	1.11	0.12	_	_	_		47
	11.0				0.33' - <2'	0.44	0.64	0.54	0.24	0.24	0.39	0.44		
				4	2' - <3'	0.44	0.64	0.54	0.12	-	-	0.44		58
					3' - <5'	0.39	0.57	0.52	0.12	_	_	_		52
10' x 6'	10	10	10	to	5' - 10'	0.37	0.37	0.52	0.12	_	_	_		52
10 x 0	10	10	10		15'	0.51	0.48	0.52	0.12	_	_	_		47
				12						-	-	-		47
	10.5	10.5	1.0	0.1	20'	0.67	0.83	0.80	0.12	-	-	-		
	10.5	10.5	10	8 to	25'	0.79	1.02	0.99	0.12	-	-	-		47
	11.5	12	10	12	30'	0.85	1.17	1.14	0.12	-	-	-		47
				4	0.33' - <2'	0.43	0.66	0.57	0.24	0.24	0.38	0.43		-
					2' - <3'	0.43	0.66	0.57	0.12	-	-	-		58
				to	3' - <5'	0.38	0.59	0.55	0.12	-	-	-		58
10' x 7'	10	10	10		5' - 10'	0.37	0.50	0.54	0.12	-	-	-		52
				12	15'	0.52	0.66	0.65	0.12	-	-	-	5	47
				12	20'	0.67	0.87	0.85	0.12	-	-	-	Note	47
	10.5	10.5	10	8 to	25'	0.79	1.07	1.04	0.12	-	-	-	×	47
	11.5	12	10	12	30'	0.84	1.22	1.19	0.12	-	-	-	General	47
				4	0.33' - <2'	0.43	0.68	0.60	0.24	0.24	0.38	0.43	Jer	-
				4	2' - <3'	0.43	0.68	0.60	0.12	_	_	-	Ge.	64
				4 -	3' - <5'	0.38	0.62	0.57	0.12	_	-	-	See	58
10' x 8'	10	10	10	to	5' - 10'	0.38	0.52	0.57	0.12	-	-	-	Se	52
					15'	0.53	0.69	0.68	0.12	_	_	-		47
				12	20'	0.68	0.91	0.89	0.12	_	_	-		47
	10.5	10.5	10	8 to	25'	0.81	1.12	1.09	0.12	_	-	-		47
	11.5	12	10	12	30'	0.86	1.27	1.25	0.12	_	_	_		47
	1				0.33' - <2'	0.43	0.70	0.62	0.24	0.24	0.38	0.43		_
				4	2' - <3'	0.43	0.70	0.62	0.12	-	-	- 0.75		70
					3' - <5'	0.43	0.70	0.62	0.12	_	_	_		64
10' > 0'	10	10	10	to	5' - 10'		0.54	0.59	0.12	_	-	_		58
10' x 9'	10	10	10							_		_		
				12	15'	0.56	0.72	0.72	0.12	-	_	-		52
	10.5	1.1	10	0.1.	20'	0.71	0.95	0.94	0.12	-	-	-		47
	10.5	11	10	8 to	25'	0.82	1.15	1.13	0.12	-	_	-		47
	11.5	12	10	12	30'	0.90	1.32	1.30	0.12	-	-	-		47
				4	0.33' - <2'	0.44	0.71	0.64	0.24	0.24	0.38	0.44		-
					2' - <3'	0.44	0.71	0.64	0.17	-	-	-		79
				to	3' - <5'	0.40	0.65	0.62	0.16	-	=	-		70
10' × 10'	10	10	10		5' - 10'	0.44	0.56	0.61	0.15	-	-	-		64
				12	15'	0.60	0.75	0.76	0.12	-	-	-		52
			<u> </u>	12	20'	0.76	0.99	0.99	0.12	-	-	-		52
	10.5	11	10	8 to	25'	0.86	1.20	1.18	0.12	-	-	-		47
	11.5	12	10	12	30'	0.97	1.36	1.35	0.13	-	-	-		47

- See Sheet 1 for Reinforcing Details and dimension locations.
 See Sheet 2 for General Notes.
 See Sheet 14 for WWR Bending Diagram.

≥ DESCRIPTION: LAST REVISION 07/01/13



TAB	LE 7	- STA	ANDAF	RD PRE	CAST BOX	CULVE	RT D	ESIG	NS (2	" COV	/ER) -	· 11' S	PANS	
SPAN x RISE	SLAB	/ WAL	LTHIC	KNESS	DESIGN			R	EINFOR			15		As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(=,)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.51	0.57	0.47	0.27	0.27	0.45	0.48		_
				,	2' - <3'	0.51	0.57	0.47	0.14	-	-	-		62
				to	3' - <5'	0.48	0.57	0.46	0.14	-	-	_		62
11' x 4'	11	11	11		5' - 10'	0.47	0.50	0.50	0.14	-	-	-		55
				12	15'	0.59	0.58	0.56	0.14	-	-	-		55
					20'	0.77	0.77	0.74	0.14	-	-	-		55
	11.5	11.5	11	8 to	25'	0.92	0.95	0.91	0.14	-	-	-		55
	13	13	11	12	30'	0.94	1.09	1.06	0.14	-	-	-		55
				4	0.33' - <2'	0.45	0.62	0.52	0.27	0.27	0.41	0.45		-
				'	2' - <3'	0.45	0.62	0.52	0.14	-	-	-		62
				to	3' - <5'	0.42	0.58	0.51	0.14	-	-	-		55
11' x 6'	11	11	11		5' - 10'	0.43	0.56	0.56	0.14	-	-	-		55
				12	15'	0.54	0.65	0.64	0.14	-	-	-		50
				12	20'	0.70	0.86	0.83	0.14	-	-	-		50
	11.5	11.5	11	8 to	25'	0.83	1.07	1.03	0.14	-	-	-		50
	13	13	11	12	30'	0.85	1.22	1.19	0.14	-	-	-		50
				4	0.33' - <2'	0.42	0.67	0.57	0.27	0.27	0.39	0.43	5	-
				4	2' - <3'	0.43	0.67	0.57	0.14	-	-	-	te	62
				to	3' - <5'	0.39	0.63	0.56	0.14	-	-	-	Note	62
11' x 8'	11	11	11	ιο	5' - 10'	0.43	0.60	0.61	0.14	-	-	-		55
				12	15'	0.54	0.72	0.71	0.14	-	-	_	General	50
				12	20'	0.70	0.94	0.92	0.14	-	-	-	99)	50
	11.5	11.5	11	8 to	25'	0.82	1.16	1.13	0.14	-	-	-	See	50
	13	13	11	12	30'	0.86	1.32	1.30	0.14	-	-	-	Se	50
				4	0.33' - <2'	0.44	0.71	0.62	0.27	0.27	0.38	0.44		-
				4	2' - <3'	0.44	0.71	0.62	0.14	-	-	-		75
				+0	3' - <5'	0.41	0.67	0.61	0.14	-	-	-		69
11' × 10'	11	11	11	to	5' - 10'	0.47	0.64	0.66	0.14	-	-	-		62
				12	15'	0.59	0.78	0.78	0.14	-	-	-		55
				12	20'	0.75	1.03	1.01	0.14	-	-	-		50
	11.5	12	11	8 to	25'	0.85	1.24	1.22	0.14	-	-	-		50
	13	13.5	11	12	30'	0.91	1.40	1.39	0.14	-	-	-		50
				4	0.33' - <2'	0.45	0.72	0.64	0.27	0.27	0.39	0.45		_
				4	2' - <3'	0.45	0.72	0.64	0.18	_	_	_		86
				+ -	3' - <5'	0.42	0.69	0.63	0.18	_	_	_		75
11' × 11'	11	11	11	to	5' - 10'	0.51	0.66	0.69	0.16	-	-	-		69
				12	15'	0.63	0.81	0.82	0.14	-	-	_		55
				12	20'	0.80	1.07	1.06	0.14	-	-	-		55
	11.5	12	11	8 to	25'	0.91	1.29	1.27	0.14	-	-	-		50
	13	13.5	11	12	30'	0.99	1.44	1.44	0.14	-	_	_		50
				_						l				

TAB	LE 8	- STA	NDAF	RD PRE	CAST BOX (CULVE	RT D	ESIGI	V <i>S (2</i> '	" COV	′ER) -	· 12' S	PANS	5
SPAN x RISE (S) (R)	SLAE TOP	B / WAL		KNESS HAUNCH	DESIGN EARTH COVER			R	EINFOR (s	RCEMEN q. in./F		15		As1 EXT LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE				•	,	Í			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.52	0.57	0.45	0.29	0.29	0.47	0.49		_
				7	2' - <3'	0.52	0.57	0.45	0.15	-	_	-		73
				to	3' - <5'	0.50	0.54	0.45	0.15	-	-	-		66
12' x 4'	12	12	12		5' - 10'	0.50	0.52	0.52	0.15	-	-	-		66
				12	15'	0.63	0.61	0.59	0.15	-	-	-		59
					20'	0.82	0.81	0.77	0.15	-	-	-		59
	12.5	12.5	12	8 to	25'	0.99	0.99	0.95	0.15	-	-	-		59
	14	14	12	12	30'	1.03	1.15	1.11	0.15	-	-	-		59
				4	0.33' - <2'	0.47	0.62	0.51	0.29	0.29	0.42	0.46		-
					2' - <3'	0.47	0.62	0.51	0.15	-	-	-		66
				to	3' - <5'	0.45	0.60	0.51	0.15	-	-	-		59
12' x 6'	12	12	12		5' - 10'	0.47	0.59	0.59	0.15	-	-	-		59
				12	15'	0.57	0.68	0.66	0.15	-	-	-		53
					20'	0.74	0.90	0.86	0.15	-	-	-		53
	12.5	12.5	12	8 to	25'	0.88	1.11	1.06	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.27	1.24	0.15	-	-	-		53
				4	0.33' - <2'	0.44	0.67	0.56	0.29	0.29	0.40	0.44	7	-
					2' - <3'	0.44	0.67	0.56	0.15	-	-	-	Note	66
				to	3' - <5'	0.41	0.64	0.56	0.15	-	-	-		59
12' x 8'	12	12	12		5' - 10'	0.45	0.63	0.64	0.15	-	-	-	General	59
				12	15'	0.56	0.75	0.73	0.15	-	-	-	ine	53
					20'	0.72	0.98	0.95	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.85	1.20	1.16	0.15	-	-	-	See	53
	14	14.5	12	12	30'	0.89	1.38	1.35	0.15	-	-	-	S	53
				4	0.33' - <2'	0.44	0.71	0.60	0.29	0.29	0.39	0.44		_
					2' - <3'	0.44	0.71	0.60	0.15	-	-	-		73
				to	3' - <5'	0.42	0.68	0.60	0.15	-	-	-		66
12' × 10'	12	12	12		5' - 10'	0.47	0.67	0.69	0.15	-	-	-		59
				12	15'	0.59	0.81	0.81	0.15	-	-	-		53
					20'	0.75	1.06	1.04	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.87	1.30	1.26	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.47	1.45	0.15	-	-	-		53
				4	0.33' - <2'	0.46	0.74	0.64	0.29	0.29	0.40	0.46		
		1			2' - <3'	0.46	0.74	0.64	0.20	-	-	-		93
				to	3' - <5'	0.42	0.72	0.64	0.20	-	-	-		80
12' x 12'	12	12	12		5' - 10'	0.54	0.71	0.74	0.18	-	-	-		73
		1		12	15'	0.66	0.87	0.89	0.15	-	-	-		59
					20'	0.83	1.14	1.13	0.15	-	-	-		59
	12.5	13	12	8 to	25'	0.96	1.39	1.37	0.15	-	-	-		53
	14	14.5	12.5	12	30'	1.05	1.56	1.56	0.15	-	-	-		53

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.



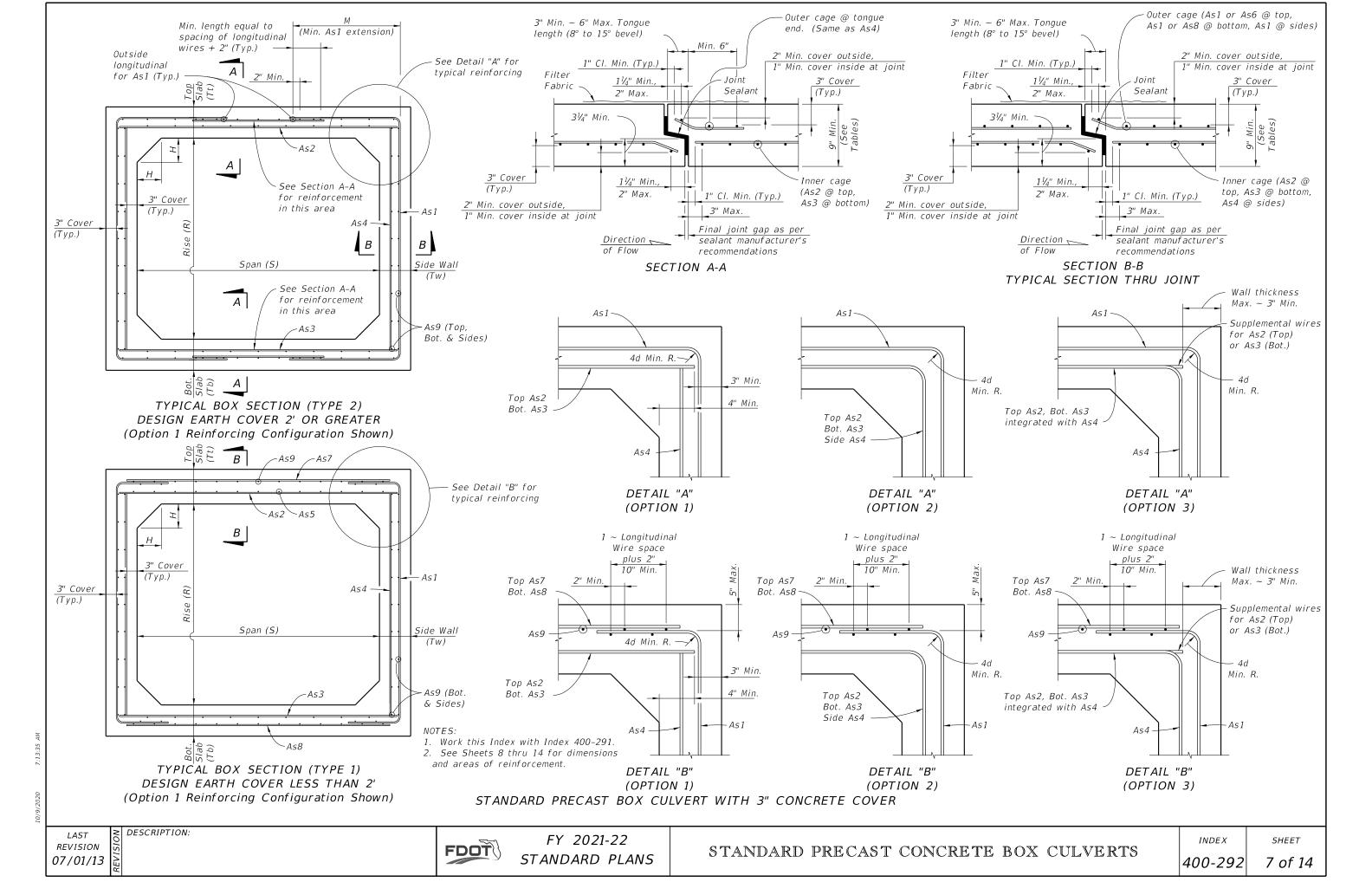


TABLE	9A -	STAN	DARD	PRECA	AST BOX CL	ILVER	T DE.	SIGNS	5 (3"	COVE	R) - 3	3' & 4	" SPA	ANS
SPAN x RISE (S) (R)	TOP (Tt)	B / WAL BOT. (Tb)	SIDE (Tw)	HAUNCH (H)	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		IS		As1 EXT LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	A58	As9	(in.)
					0.33' - <2'	0.22	0.24	0.22	0.22	0.22	0.22	0.22		_
					2' - <3'	0.11	0.23	0.22	0.11	-	-	-		31
				4	3' - <5'	0.11	0.22	0.22	0.11	-	-	-		31
					5' - 10'	0.11	0.22	0.22	0.11	-	-	-		31
3' x 3'	9	9	9	to	15'	0.11	0.22	0.22	0.11	-	-	-		31
					20'	0.13	0.22	0.22	0.11	-	-	-		31
				8	25'	0.16	0.22	0.22	0.11	-	-	-		31
					30'	0.19	0.24	0.25	0.11	-	-	-		31
					35'	0.22	0.28	0.29	0.11	-	-	-		31
					0.33' - <2'	0.22	0.32	0.24	0.22	0.22	0.22	0.22	5	_
				4	2' - <3'	0.17	0.31	0.24	0.11	_	-	-	Note	38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 3'	9	9	9	to	5' - 10'	0.13	0.22	0.22	0.11	-	-	_	General	38
					15'	0.17	0.22	0.22	0.11	-	-	-	neı	38
				8	20'	0.23	0.26	0.27	0.11	-	-	-		38
					25'	0.28	0.32	0.34	0.11	1	-	-	See	38
					30'	0.33	0.39	0.40	0.11	ı	-	-	S	38
					0.33' - <2'	0.22	0.34	0.26	022	0.22	0.22	0.22		_
				4	2' - <3'	0.17	0.33	0.26	0.11	-	-	-		38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 4'	9	9	9	to	5' - 10'	0.14	0.22	0.22	0.11	-	-	-		38
					15'	0.19	0.22	0.23	0.11	-	-	-		38
				8	20'	0.24	0.28	0.30	0.11	-	-	-		38
					25'	0.29	0.36	0.37	0.11	-	-	-		38
					30'	0.34	0.43	0.45	0.11	-	-	-		38

TABLE	9B -	STAN	DARE	PREC	AST BOX CL	ILVER	T DE.	SIGNS	5 (3"	COVE	R)	3' & 4	l' SPA	ANS
SPAN x RISE (S) (R)	SLAE TOP (Tt)	BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R	EINFOR (s	RCEMEN q. in./F		15		As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.24	0.24	0.24	0.24	0.24	0.24		-
					2' - <3'	0.12	0.24	0.24	0.24	-	_	-		31
				4	3' - <5'	0.12	0.24	0.24	0.24	-	_	-		31
					5' - 10'	0.12	0.24	0.24	0.24	-	-	-		31
3' x 3'	10	10	10	to	15'	0.12	0.24	0.24	0.24	-	-	-		31
					20'	0.12	0.24	0.24	0.24	-	_	-		31
				8	25'	0.13	0.24	0.24	0.24	-	-	-		31
					30'	0.15	0.24	0.24	0.12	-	-	-		31
					35'	0.18	0.24	0.24	0.12	-	-	-		31
					0.33' - <2'	0.24	0.26	0.24	0.24	0.24	0.24	0.24	5	-
				4	2' - <3'	0.14	0.26	0.24	0.12	-	_	_	Note	38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-	No	38
4' x 3'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-	le.	38
					15'	0.14	0.24	0.24	0.12	-	-	-	eneral	38
				8	20'	0.18	0.24	0.24	0.12	-	-	-	Ge.	38
					25'	0.22	0.26	0.27	0.12	-	-	-	ee	38
					30'	0.26	0.31	0.32	0.12	-	-	-	Sé	38
					0.33' - <2'	0.24	0.28	0.24	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.14	0.28	0.24	0.12	-	-	-		38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-		38
4' x 4'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	_	-		38
					15'	0.15	0.24	0.24	0.12	-	_	-		38
				8	20'	0.19	0.24	0.24	0.12	-	_	-		38
					25'	0.23	0.28	0.30	0.12	-	-	-		38
					30'	0.27	0.34	0.35	0.12	-	_	-		38

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

LAST	NC	DESCRIPTION:
REVISION	SI	
07/01/13	EV I	

					AST BOX CU	CVLI	I DE						3 3 7 /	
PAN x RISE S) (R)	SLAE TOP	B / WAL BOT.		KNESS HAUNCH	DESIGN EARTH COVER			R		RCEMEN g. in./F	T AREA	IS		As1 EX
5) (h)	(Tt)	(Tb)	(Tw)	HAUNCH (H)	ABOVE				(5	y. 111./ F	L.)			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	1 - 1	4.5.2	4.52	1 - 1	1 o F	1.57	1.50	450	(in.)
	(, , , , ,	(1111)	(/////	(111.7)		As 1	As2	As3	As4	As5	As7	A58	As9	, ,
					0.33' - <2'	0.27	0.39	0.37	0.22	0.22	0.22	0.27		4.5
				4	2' - <3'	0.26	0.39	0.37	0.11	-	_	_		45
EL 21	9	9	9		3' - <5'	0.19	0.24	0.25	0.11	-	-	-		36
5' x 3'	9	9	9	to	5' - 10' 15'	0.20 0.28	0.22	0.22	0.11	-	-	_		36 35
					20'	0.28	0.28	0.30	0.11	_	_	_		35
				8	25'	0.37	0.38	0.39	0.11					35
					30'	0.43	0.48	0.49	0.11	-	-	-		35
					0.33' - <2'	0.26			0.11	- 0.22	- 0.22	0.26		
					2' - <3'	0.26	0.42	0.39	0.22	0.22	0.22	0.26		45
				4	3' - <5'	0.20	0.42	0.39	0.11	_	_	_		45
5' x 4'	9	9	9	,	5' - 10'	0.19	0.20	0.27	0.11	_	_	_		36
J X 4	9	9	9	to	15'	0.27	0.22	0.23	0.11	_	_	_		35
					20'	0.27	0.42	0.33	0.11	_	_	_		35
				8	25'	0.36	0.42	0.43	0.11	_	_	_		35
					30'	0.44	0.52	0.54	0.11	-	_	_		35
					0.33' - <2'	0.33	0.63	0.65	0.11	0.22	0.22	0.27		-
					0.33' - <2' 2' - <3'	0.27	0.44	0.42	0.22	0.22	0.22			45
				4	2' - <3' 3' - <5'	0.27	0.44	0.42	0.11	-	_	_		45
5' x 5'	9	9	9	,	5' - 10'	0.22	0.27	0.26	0.11	_	_	_		45
J X J	9	9	9	to	15'	0.30	0.23	0.26	0.11	_	_	_		36
					20'	0.38	0.45	0.47	0.11	_	_	_		35
				8	25'	0.47	0.45	0.59	0.11	_	_	_		35
					30'	0.55	0.68	0.71	0.11	_	_	_		35
					0.33' - <2'	0.33	0.00	0.71	0.11	0.22	0.25	0.34	5	-
					2' - <3'	0.34	0.47	0.42	0.22	-	-	-		43
				4	3' - <5'	0.27	0.47	0.42	0.11	_	_	_	Note	39
6' x 3'	9	9	9		5' - 10'	0.27	0.26	0.32	0.11	_	_	_		39
0 x 3	9	9	9	to	15'	0.29	0.39	0.28	0.11	_	_	_	er ë	38
				1.2	20'	0.42	0.52	0.40	0.11	_	_	_	General	38
				12	25'	0.55	0.52	0.53	0.11	_	_	_		38
					30'	0.82	0.81	0.82	0.11	_	_	_	See	38
					0.33' - <2'	0.33	0.50	0.46	0.22	0.22	0.23	0.33		- 50
				1	2' - <3'	0.33	0.50	0.46	0.22	-	-	-		43
				4	3' - <5'	0.27	0.33	0.40	0.11	_	_	_		39
6' x 4'	9	9	9	t o	5' - 10'	0.28	0.29	0.31	0.11	_	_	_		39
J / /				to	15'	0.40	0.43	0.45	0.11	_	_	_		38
				12	20'	0.52	0.43	0.43	0.11	_	_			38
				12	25'	0.65	0.73	0.74	0.11	_	_	_		38
					30'	0.78	0.88	0.90	0.11	_	_	_		38
					0.33' - <2'	0.33	0.52	0.49	0.22	0.22	0.23	0.33		-
				4	2' - <3'	0.33	0.52	0.49	0.22	-	-			43
				4	3' - <5'	0.27	0.35	0.49	0.11	_	_	_		43
6' x 5'	9	9	9	to	5' - 10'	0.27	0.31	0.34	0.11	_	_	_		39
J , J				"	15'	0.41	0.46	0.49	0.11	_	_	_		38
				12	20'	0.53	0.62	0.43	0.11	_	_	_		38
				12	25'	0.66	0.78	0.80	0.11	_	_	_		38
					30'	0.78	0.95	0.97	0.11	_	_	_		38
					0.33' - <2'	0.78	0.55	0.51	0.11	0.22	0.24	0.34		
				1	2' - <3'	0.34	0.54	0.51	0.22	-	-	-		52
				4	3' - <5'	0.29	0.37	0.39	0.11	_	_	_		52
6' x 6'	9	9	9	t o	5' - 10'	0.29	0.34	0.39	0.11	_	_	_		43
0 10	9	9)	to	15'	0.44	0.50	0.53	0.11	_	_	_		39
				13	20'	0.44	0.66	0.70	0.11	_	_	_		39
				12	25'	0.70	0.84	0.70	0.11	_	_	_		38
					30'	0.70	1.02	1.05	0.11	_	_	_		38
	1			l	JU	0.05	1.02	1.00	0.11					

IADLL	IUB -	SIAN	IDAKL) PREC.	AST BOX CU	JLVER	RI DE	SIGN.	5 (3"	COVE	ER) -	5' & 6	5' SP.	4 <i>NS</i>
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR			15		As1 EXT
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M)
(FL.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.33	0.32	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.22	0.33	0.32	0.12	-	-	-		45
					3' - <5'	0.16	0.24	0.24	0.12	-	-	-		36
5' x 3'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-		36
					15'	0.23	0.24	0.24	0.12	-	-	-		35
				12	20'	0.29	0.30	0.31	0.12	-	-	-		35
					25'	0.36	0.38	0.39	0.12	-	-	-		35
					30'	0.43	0.46	0.47	0.12	-	-	-		35
					0.33' - <2'	0.24	0.35	0.34	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.22	0.35	0.34	0.12	-	-	-		45
					3' - <5'	0.15	0.24	0.24	0.12	-	-	-		45
5' x 4'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-		36
					15'	0.22	0.25	0.27	0.12	-	-	-		35
				12	20'	0.29	0.33	0.34	0.12	-	-	-		35
					25'	0.36	0.41	0.43	0.12	-	-	-		35
					30'	0.42	0.50	0.51	0.12	-	-	-		35
					0.33' - <2'	0.24	0.37	0.36	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.21	0.37	0.36	0.12	-	-	-		45
					3' - <5'	0.16	0.24	0.25	0.12	-	_	-		45
5' x 5'	10	10	10	to	5' - 10'	0.17	0.24	0.24	0.12	-	-	-		45
					15'	0.24	0.27	0.29	0.12	-	-	-		36
				12	20'	0.30	0.36	0.38	0.12	-	-	-		35
					25'	0.37	0.44	0.47	0.12	-	-	-		35
					30'	0.44	0.53	0.56	0.12	-	-	-		35
					0.33' - <2'	0.28	0.40	0.36	0.24	0.24	0.24	0.28	5	_
				4	2' - <3'	0.28	0.40	0.36	0.12	-	-	-	te	43
					3' - <5'	0.22	0.26	0.28	0.12	-	-	-	Note	39
6' x 3'	10	10	10	to	5' - 10'	0.24	0.24	0.24	0.12	-	-	-	General	39
					15'	0.34	0.31	0.32	0.12	-	-	-	ner	38
				12	20'	0.44	0.41	0.42	0.12	-	-	-	99)	38
					25'	0.54	0.52	0.53	0.12	-	-	-	See	38
					30'	0.64	0.63	0.64	0.12	-	-	-	Se	38
					0.33' - <2'	0.27	0.42	0.39	0.24	0.24	0.24	0.27		_
				4	2' - <3'	0.27	0.42	0.39	0.12	-	-	-		43
					3' - <5'	0.21	0.28	0.30	0.12	-	-	-		39
6' x 4'	10	10	10	to	5' - 10'	0.23		0.25	0.12	-	-	-		39
					15'	0.32	0.34	0.35	0.12	-	-	-		38
				12	20'	0.42	0.45	0.47	0.12	-	-	-		38
				_	25'	0.51	0.56	0.58	0.12	-	-	-		38
					30'	0.61	0.68	0.70	0.12	-	_	-		38
					0.33' - <2'	0.26	0.44	0.42	0.24	0.24	0.24	0.26		_
				4	2' - <3'	0.26	0.44	0.42	0.12	_	_	-		43
				'	3' - <5'	0.22	0.30	0.33	0.12	_	_	-		43
6' x 5'	10	10	10	to	5' - 10'	0.24	0.25	0.27	0.12	-	_	-		39
-		_			15'	0.33	0.36	0.39	0.12	-	_	-		38
				12	20'	0.42	0.48	0.51	0.12	-	_	-		38
				'-	25'	0.52	0.61	0.63	0.12	-	_	-		38
					30'	0.61	0.74	0.76	0.12	_	_	-		38
					0.33' - <2'	0.27	0.46	0.44	0.24	0.24	0.24	0.27		-
				4	2' - <3'	0.27	0.46	0.44	0.12	-		-		52
				+	3' - <5'	0.23	0.31	0.34	0.12	_	_	_		52
6' x 6'	10	10	10	to	5' - 10'	0.25	0.27	0.30	0.12	_	_	_		43
0 1 0	1		'	to	15'	0.25	0.39	0.42	0.12	_	_	_		39
				12	20'	0.35	0.52	0.42	0.12	_	_	_		39
				12	25'	0.43	0.52	0.55	0.12	_	_	_		38
					30'	0.64	0.03	0.81	0.12	_	_	_		38
				l	30	0.04	1 0.70	0.01	0.12	_		_		

≥ DESCRIPTION: LAST REVISION 07/01/13

TABI	LE 11A	- ST	ANDA	RD PRE	CAST BOX	CULV	ERT .	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPANS	5
SPAN x RISE		/ WAL			DESIGN EARTH COVER			R	EINFOR			15		As1 EXT. LENGTH
(S) (R)	TOP (Tt)	B0T. (Tb)	(Tw)	HAUNCH (H)	ABOVE				(5	q. in./F	(.)			(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.42	0.58	0.52	0.22	0.22	0.31	0.42	7133	_
				4	2' - <3'	0.42	0.58	0.51	0.11	_	-	_		43
				,	3' - <5'	0.36	0.41	0.44	0.11	_	-	-		43
7' x 4'	9	9	9	to	5' - 10'	0.39	0.40	0.39	0.11	-	-	-	1	43
					15'	0.56	0.56	0.58	0.11	-	-	-	1	41
				12	20'	0.74	0.76	0.77	0.11	-	-	-	1	41
					25'	0.92	0.97	0.97	0.11	-	-	-	1	41
	9	9.5	9	7 to 12	30'	1.09	1.18	1.10	0.11	-	-	-	1	41
					0.33' - <2'	0.41	0.61	0.55	0.22	0.23	0.30	0.41	1	_
				4	2' - <3'	0.41	0.61	0.55	0.11	-	-	-		47
					3' - <5'	0.37	0.43	0.47	0.11	-	-	-		43
7' x 5'	9	9	9	to	5' - 10'	0.39	0.41	0.43	0.11	-	-	-		43
					15'	0.56	0.61	0.63	0.11	-	-	-	5	41
				12	20'	0.73	0.82	0.83	0.11	-	-	-	Note	41
					25'	0.90	1.04	1.06	0.11	-	-	-	No	41
	9	9.5	9	7 to 12	30'	1.06	1.26	1.19	0.11	-	-	-	General	41
					0.33' - <2'	0.42	0.63	0.58	0.22	0.24	0.30	0.42	neı	-
				4	2' - <3'	0.42	0.63	0.58	0.11	-	-	-		59
					3' - <5'	0.38	0.45	0.50	0.11	_	-	_	See	47
7' x 6'	9	9	9	to	5' - 10'	0.41	0.44	0.47	0.11	-	-	-	S	43
					15'	0.57	0.65	0.68	0.11	-	-	-		41
				12	20'	0.75	0.87	0.90	0.11	-	-	-		41
					25'	0.93	1.11	1.13	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.07	1.35	1.27	0.11	-	-	-		41
					0.33' - <2'	0.44	0.66	0.61	0.22	0.25	0.31	0.44		
				4	2' - <3'	0.44	0.65	0.61	0.11	-	-	-		59
					3' - <5'	0.41	0.47	0.52	0.11	-	-	_		59
7' x 7'	9	9	9	to	5' - 10'	0.44	0.47	0.52	0.11	-	-	_		47
					15'	0.62	0.69	0.74	0.11	-	-	_		43
				12	20'	0.80	0.93	0.97	0.11	_	-	_		43
					25'	0.99	1.18	1.22	0.11	-	-	-		43
	9	9.5	9	7 to 12	30'	1.12	1.43	1.36	0.11	-	-	-		41

TABI	LE 11B	- ST	ANDA	RD PRI	CAST BOX	CULV	ERT	DESIG	SNS (3" CO	VER)	- 7' 5	SPANS	5
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.33	0.49	0.44	0.24	0.24	0.24	0.33		-
				4	2' - <3'	0.33	0.49	0.44	0.12	-	-	-		43
					3' - <5'	0.29	0.35	0.38	0.12	-	-	-		43
7' x 4'	10	10	10	to	5' - 10'	0.31	0.30	0.31	0.12	-	-	-		43
					15'	0.44	0.44	0.45	0.12	-	-	-		41
				12	20'	0.58	0.59	0.60	0.12	-	-	-		41
					25'	0.71	0.74	0.75	0.12	-	-	-		41
					30'	0.85	0.91	0.91	0.12	-	-	-		41
					0.33' - <2'	0.32	0.51	0.47	0.24	0.24	0.24	0.32		-
				4	2' - <3'	0.32	0.51	0.47	0.12	-	-	-		47
					3' - <5'	0.29	0.37	0.41	0.12	-	-	-		43
7' x 5'	10	10	10	to	5' - 10'	0.31	0.32	0.35	0.12	-	-	_		43
					15'	0.44	0.47	0.50	0.12	-	-	-	5	41
				12	20'	0.57	0.63	0.65	0.12	-	-	-	te	41
					25'	0.70	0.80	0.82	0.12	-	-	-	Note	41
					30'	0.84	0.97	0.99	0.12	-	-	-	le.	41
					0.33' - <2'	0.33	0.53	0.50	0.24	0.24	0.24	0.33	General	-
				4	2' - <3'	0.33	0.53	0.50	0.12	-	-	-	99)	59
					3' - <5'	0.30	0.38	0.43	0.12	-	-	-	See	47
7' x 6'	10	10	10	to	5' - 10'	0.33	0.35	0.38	0.12	-	-	-	Se	43
					15'	0.45	0.51	0.54	0.12	-	-	-		41
				12	20'	0.58	0.68	0.70	0.12	-	-	-		41
					25'	0.72	0.85	0.88	0.12	-	-	-		41
					30'	0.85	1.04	1.06	0.12	-	-	-		41
					0.33' - <2'	0.35	0.55	0.52	0.24	0.24	0.24	0.35		_
				4	2' - <3'	0.35	0.55	0.52	0.12	-	-	-		59
					3' - <5'	0.32	0.40	0.46	0.12	_	-	-		59
7' x 7'	10	10	10	to	5' - 10'	0.35	0.37	0.41	0.12	-	-	-		47
					15'	0.48	0.54	0.58	0.12	-	-	-		43
				12	20'	0.62	0.72	0.76	0.12	-	-	-		43
					25'	0.76	0.90	0.94	0.12	-	-	-		43
					30'	0.90	1.10	1.13	0.12	-	-	-		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: REVISION 07/01/13



TABL	E 124	\ - ST	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (3" CC	VER)	- 8' .	SPAN	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	l	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(F+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.52	0.66	0.57	0.22	0.24	0.42	0.52		-
				7	2' - <3'	0.52	0.66	0.57	0.11	-	-	-		50
				to	3' - <5'	0.48	0.49	0.52	0.11	-	-	-		50
8' x 4'	9	9	9	10	5' - 10'	0.52	0.48	0.49	0.11	-	-	-		45
				12	15'	0.75	0.72	0.72	0.11	-	-	-		41
				12	20'	1.00	0.98	0.97	0.11	_	-	-		41
	9	9.5	9	8 to	25'	1.25	1.24	1.14	0.11	-	-	-		41
	10	10.5	9	12	30'	1.31	1.29	1.21	0.11	-	-	-		41
				4	0.33' - <2'	0.51	0.69	0.60	0.22	0.25	0.40	0.51		-
				7	2' - <3'	0.51	0.69	0.60	0.11	-	-	-		50
				to	3' - <5'	0.46	0.52	0.56	0.11	-	-	-		45
8' x 5'	9	9	9	10	5' - 10'	0.51	0.51	0.53	0.11	_	-	_		45
				12	15'	0.74	0.77	0.78	0.11	-	-	-		41
				12	20'	0.97	1.05	1.05	0.11	1	-	-		41
	9	9.5	9	8 to	25'	1.20	1.33	1.23	0.11	_	-	-		41
	10	10.5	9	12	30'	1.26	1.38	1.30	0.11	_	-	-		41
				1	0.33' - <2'	0.51	0.72	0.64	0.22	0.26	0.39	0.51	5	-
				4	2' - <3'	0.51	0.72	0.64	0.11	-	-	-	Note	50
				+ 0	3' - <5'	0.47	0.55	0.59	0.11	-	-	-		50
8' x 6'	9	9	9	to	5' - 10'	0.52	0.55	0.58	0.11	-	-	-	General	45
				1 2	15'	0.74	0.83	0.85	0.11	_	-	-	ner	41
				12	20'	0.97	1.12	1.13	0.11	-	-	-	(<i>e</i>)	41
	9	9.5	9	8 to	25'	1.18	1.42	1.32	0.11	-	-	-	See	41
	10	10.5	9	12	30'	1.26	1.46	1.39	0.11	-	-	-	Se	41
				4	0.33' - <2'	0.52	0.74	0.67	0.22	0.26	0.40	0.52		-
				4	2' - <3'	0.52	0.74	0.67	0.11	-	-	-		55
				<i>t</i> ~	3' - <5'	0.49	0.57	0.62	0.11	-	-	-		55
8' x 7'	9	9	9	to	5' - 10'	0.55	0.59	0.63	0.11	-	-	-		50
				1 7	15'	0.77	0.88	0.91	0.11	_	-	_		41
				12	20'	1.01	1.19	1.21	0.11	-	-	_		41
	9	9.5	9	8 to	25'	1.21	1.51	1.41	0.11	-	-	-		41
	10	10.5	9	12	30'	1.31	1.53	1.47	0.11	-	-	-		41
				4	0.33' - <2'	0.55	0.77	0.70	0.22	0.27	0.41	0.55		-
				4	2' - <3'	0.55	0.77	0.70	0.13	_	-	_		65
					3' - <5'	0.53	0.59	0.64	0.12	_	-	_		65
8' x 8'	9	9	9	to	5' - 10'	0.60	0.63	0.68	0.11	-	-	-		55
				1.3	15'	0.83	0.93	0.98	0.11	_	-	-		45
				12	20'	1.08	1.26	1.29	0.11	_	-	_		45
	9	9.5	9	8 to	25'	1.28	1.59	1.50	0.11	-	_	_		41
	10	10.5	9	12	30'	1.41	1.61	1.55	0.11	_	-	-		41
											1		L	

TABL	.E 12E	3 - ST	AND	ARD PR	ECAST BOX	CULV	/ERT	DESIG	GNS (3" CC	VER)	- 8' .	SPAN.	S
SPAN x RISE				KNESS	DESIGN			R	EINFOF			15		As1 EX
(S) (R)	TOP	BOT.		HAUNCH					(5	q. in./F	t.)			LENGTI
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB			1			ı	1	1	(M) (in.)
(1 2.)	(111.)	(111.)	(111.)	(111.)		As1	As2	As3	As4	As5	As7	A58	A59	(111.)
					0.33' - <2'	0.42	0.56	0.49	0.24	0.24	0.32	0.41		
				4	2' - <3'	0.42	0.56	0.49	0.12	-	-	-		50
					3' - <5'	0.38	0.42	0.46	0.12	-	-	-		50
8' x 4'	10	10	10	to	5' - 10'	0.41	0.38	0.39	0.12	-	-	-		45
					15'	0.59	0.56	0.57	0.12	-	-	-		41
				12	20'	0.78	0.75	0.76	0.12	-	-	-		41
					25'	0.97	0.96	0.96	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.15	1.16	1.10	0.12	-	-	-		41
					0.33' - <2'	0.40	0.58	0.52	0.24	.034	0.31	0.40		
				4	2' - <3'	0.40	0.58	0.52	0.12	-	-	-		50
					3' - <5'	0.37	0.45	0.48	0.12	-	-	-		45
8' x 5'	10	10	10	to	5' - 10'	0.41	0.41	0.43	0.12	-	-	-		45
					15'	0.58	0.60	0.62	0.12	-	-	-		41
				12	20'	0.76	0.81	0.81	0.12	-	-	-		41
					25'	0.94	1.03	1.03	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.10	1.24	1.24	0.12	-	-	-		41
					0.33' - <2'	0.40	0.60	0.55	0.24	0.24	0.30	0.40	5	_
				4	2' - <3'	0.40	0.60	0.55	0.12	-	-	-	Note	50
					3' - <5'	0.37	0.47	0.51	0.12	-	-	-		50
8' x 6'	10	10	10	to	5' - 10'	0.42	0.43	0.46	0.12	-	-	-	General	45
					15'	0.58	0.64	0.67	0.12	-	-	-	ene	41
				12	20'	0.76	0.86	0.88	0.12	-	-	-		41
					25'	0.94	1.09	1.11	0.12	-	-	-	See	41
	10	10.5	10	8 to 12	30'	1.09	1.32	1.26	0.12	-	-	-) ₂	41
					0.33' - <2'	0.41	0.63	0.58	0.24	0.24	0.30	0.41		_
				4	2' - <3'	0.41	0.63	0.58	0.12	-	-	-		55
					3' - <5'	0.39	0.49	0.53	0.12	-	-	-		55
8' x 7'	10	10	10	to	5' - 10'	0.44	0.46	0.50	0.12	-	-	-		50
					15'	0.61	0.68	0.72	0.12	-	-	-		45
				12	20'	0.78	0.91	0.94	0.12	-	-	-		41
					25'	0.97	1.16	1.18	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.11	1.40	1.34	0.12	-	-	-		41
					0.33' - <2'	0.44	0.64	0.60	0.24	0.24	0.31	0.44		
		1		4	2' - <3'	0.44	0.64	0.60	0.12	-	_	-		65
]			3' - <5'	0.42	0.51	0.56	0.12	-	-	-		65
8' x 8'	10	10	10	to	5' - 10'	0.47	0.50	0.55	0.12	-	-	-		55
		1			15'	0.65	0.72	0.77	0.12	-	-	-		45
		1		12	20'	0.84	0.96	1.01	0.12	-	-	-		45
					25'	1.03	1.22	1.26	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.16	1.47	1.42	0.12	-	-	-		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: REVISION 07/01/13



TABLI	E 13A	- <i>ST</i>	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (.	3" CC	VER)	- 9' 5	SPAN	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	ıS		As1 EXT.
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
4	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.62	0.78	0.65	0.22	0.26	0.52	0.61		-
				4	2' - <3'	0.62	0.78	0.65	0.11	-	-	-		54
				to	3' - <5'	0.58	0.63	0.61	0.11	-	-	-		49
9' x 5'	9	9	9	12	5' - 10'	0.65	0.63	0.64	0.11	-	-	-		49
					15'	0.95	0.96	0.95	0.11	-	-	-		44
	9	9	9	8	20'	1.26	1.32	1.28	0.11	-	-	-		44
	10	10.5	9	to	25'	1.39	1.41	1.32	0.11	-	-	-		44
	11	11.5	9	12	30'	1.46	1.50	1.42	0.11	-	-	-		44
					0.33' - <2'	0.60	0.81	0.69	0.22	0.27	0.51	0.60		-
				4	2' - <3'	0.60	0.81	0.69	0.11	-	-	-		54
				to	3' - <5'	0.56	0.66	0.65	0.11	-	_	_		49
9' x 6'	9	9	9	12	5' - 10'	0.65	0.68	0.69	0.11	-	-	-		49
					15'	0.94	1.03	1.02	0.11	-	-	-		44
	9	9	9	8	20'	1.25	1.40	1.38	0.11	-	-	-		44
l L	10	10.5	9	to	25'	1.37	1.49	1.40	0.11	1	1	1		44
	11	11.5	9	12	30'	1.44	1.58	1.50	0.11	ı	ı	1		44
					0.33' - <2'	0.61	0.84	0.72	0.22	0.28	0.51	0.61	5	-
				4	2' - <3'	0.61	0.83	0.72	0.11	1	1	1	Note	59
				to	3' - <5'	0.58	0.69	0.68	0.11	-	-	1		54
9' x 7'	9	9	9	12	5' - 10'	0.67	0.73	0.75	0.11	1	ı	1	General	49
					15'	0.96	1.09	1.10	0.11	1	-	1	neı	44
	9	9	9	8	20'	1.27	1.49	1.47	0.11	1	-	1	<i>Ge</i>	44
	10	10.5	9	to	25'	1.38	1.57	1.48	0.11	-	-	-	See	44
	11	11.5	9	12	30'	1.49	1.70	1.58	0.11	-	_	-	Sé	44
	9	9.5	9		0.33' - <2'	0.60	0.85	0.73	0.22	0.29	0.52	0.53		-
				4	2' - <3'	0.64	0.86	0.76	0.12	-	-	1		59
				to	3' - <5'	0.62	0.72	0.72	0.11	-	-	1		59
9' x 8'	9	9	9	12	5' - 10'	0.71	0.77	0.81	0.11	-	-	-		54
					15'	1.01	1.16	1.17	0.11	-	_	1		44
	9	9.5	9	8	20'	1.27	1.56	1.45	0.11	ı	ı	1		44
	10	10.5	9	to	25'	1.45	1.65	1.57	0.11	-	-	1		44
	11	11.5	9	12	30'	1.59	1.72	1.66	0.11	-	-	1		44
	9	9.5	9		0.33' - <2'	0.68	0.88	0.76	0.22	0.29	0.55	0.57		-
				4	2' - <3'	0.68	0.88	0.78	0.18	-	_	-		72
				to	3' - <5'	0.68	0.75	0.78	0.18	-	_	-		72
9' x 9'	9	9	9	12	5' - 10'	0.79	0.82	0.88	0.17	-	-	1		59
					15'	1.11	1.22	1.26	0.13	-	-	1		49
	9	9.5	9	8	20'	1.37	1.64	1.54	0.13	-	-	-		49
	10	10.5	9	to	25'	1.56	1.73	1.65	0.13	-	-	-		44
. ⊢	11	11.5	9.5	12	30'	1.56	1.73	1.68	0.12					44

					ECAST BOX	CULV	CKI						SPAN.	
SPAN x RISE			LTHIC		DESIGN			R	EINFOR			15		AS1 EX
(S) (R)	TOP	B0T. (Tb)	SIDE (Tw)	HAUNCH	EARTH COVER ABOVE				(5	q. in./F	L.)			LENGT (M)
(Ft.)	(Tt) (in.)	(in.)	(in.)	(H) (in.)	TOP SLAB	1 - 1	1.52	4.5.2	151	1 o E	4.57	450	1.50	(in.,
, ,	(, , , , ,	(,,,,,	(, , , , ,	(/////	0.33' - <2'	As1 0.49	<i>As2</i> 0.65	<i>As3</i> 0.57	0.24	As5 0.24	<i>As7</i> 0.40	0.48	As9	_
				4	2' - <3'	0.49	0.65	0.57	0.24	0.24	-	0.48		54
					3' - <5'	0.49	0.54	0.57	0.12	_	_	_		49
9' x 5'	10	10	10	to	5' - 10'	0.52	0.50	0.51	0.12	_	_	_		49
3 % 3					15'	0.75	0.74	0.75	0.12	_	_	_		44
				12	20'	0.98	1.01	1.00	0.12	_	_	_		44
	10	10.5	10	8 to	25'	1.21	1.27	1.19	0.12	_	_	_		44
	11	11.5	10	12	30'	1.30	1.36	1.30	0.12	-	-	-		44
					0.33' - <2'	0.48	0.68	0.60	0.24	0.24	0.39	0.48		_
				4	2' - <3'	0.48	0.68	0.60	0.12	-	-	-		54
			1		3' - <5'	0.45	0.57	0.56	0.12	-	-	-	1	49
9' x 6'	10	10	10	to	5' - 10'	0.52	0.53	0.56	0.12	-	-	-		49
					15'	0.74	0.79	0.81	0.12	-	-	-		44
				12	20'	0.97	1.07	1.07	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.18	1.35	1.28	0.12	-	-	-		44
	11	11.5	10	12	30'	1.27	1.44	1.38	0.12	-	-	-		44
					0.33' - <2'	0.49	0.70	0.63	0.24	0.24	0.39	0.49	72	-
				4	2' - <3'	0.49	0.70	0.63	0.12	-	-	-	Note	59
	1.0	10	1.0	4.	3' - <5'	0.46	0.59	0.59	0.12	-	-	-		54
9' x 7'	10	10	10	to	5' - 10'	0.54	0.57	0.60	0.12	-	-	-	General	49
				12	15'	0.75	0.84	0.86	0.12	-	-	-	ne	44
					20'	0.98	1.13	1.14	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.18	1.43	1.36	0.12	-	-	-	See	44
	11	11.5	10	12	30'	1.28	1.52	1.46	0.12	-	-	-	S	44
				4	0.33' - <2'	0.51	0.72	0.65	0.24	0.24	0.39	0.51		_
				"	2' - <3'	0.51	0.72	0.65	0.12	_	_	-		59
	10	10	10	to	3' - <5'	0.49	0.61	0.62	0.12	-	-	-		59
9' x 8'		10	10		5' - 10'	0.57	0.60	0.65	0.12	-	-	-		54
				12	15'	0.79	0.89	0.92	0.12	-	-	-		44
	1.0	10.5	1.0		20'	1.02	1.20	1.22	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.21	1.50	1.44	0.12	_	_	_		44
	11	11.5	10	12	30'	1.33	1.59	1.54	0.12		- 0.41	- 0.54	-	
				4	0.33' - <2'	0.54	0.74	0.68	0.24	0.24	0.41	0.54		72
					2' - <3' 3' - <5'	0.54	0.74	0.68	0.15	-	-	-		72
9' x 9'	10	10	10	to	5' - 10'	0.53	0.63	0.64	0.13		-	-		72 59
9 X 9					3 - 10 15'	0.62 0.85	0.64	0.70	0.12	-	-	_	-	49
				12	20'	1.09	1.26	1.29	0.12	_	-	_	-	49
	10	10.5	10	8 to	25'	1.28	1.56	1.52	0.12	_	_	_		49
	11	11.5	10	12	30'	1.42	1.66	1.66	0.12	_	_	_	-	44
	11	11.5	10	1 4	1 50	1.42	1.00	1.00	0.12	_	_	_		1 45

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

≥ DESCRIPTION:

TAB					CAST BOX	CULV	ERT [DESIG	NS (3	3" CO	VER)	- 11' S	SPANS	5
SPAN x RISE		/ WAL			DESIGN			R			TAREA	15		As1 EX
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER ABOVE				(5	q. in./F	t.)			LENGTI (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	A = 1	12	4.52	151	1 A S E	1.57	450	A59	(in.)
	(1111)	(1111)	(*****)	<u> </u>	0.33' - <2'	0.60	<i>As2</i> 0.66	As3 0.54	As4 0.27	As5 0.27	0.52	0.56	ASS	_
				4	2' - <3'	0.60	0.66	0.54	0.27	0.27	0.52	0.30	<u> </u>	62
					3' - <5'	0.60	0.61	0.53	0.14	_	_	_		62
11' × 4'	11	11	1 1 1	to	5' - 10'	0.79	0.63	0.62	0.14	_	_	_		55
				1.2	15'	1.01	0.82	0.79	0.14	-	_	_		55
				12	20'	1.34	1.11	1.06	0.14	_	_	_		55
	12	12	11	8 to	25'	1.52	1.27	1.23	0.14	-	_	-		55
	13.5	13.5	11	12	30'	1.54	1.37	1.34	0.14	-	_	-	1	50
					0.33' - <2'	0.57	0.71	0.60	0.27	0.27	0.47	0.53	1	-
				4	2' - <3'	0.56	0.71	0.60	0.14	-	_	-		62
				to	3' - <5'	0.56	0.67	0.59	0.14	-	-	-		55
11' x 6'	11	11	11	12	5' - 10'	0.73	0.71	0.72	0.14	-	-	-		55
					15'	0.92	0.92	0.91	0.14	-	-	-		50
	11	11	11	8	20'	1.21	1.25	1.21	0.14	-	-	-		50
	12	12	11	to	25'	1.37	1.43	1.39	0.14	-	-	-		50
	13.5	13.5	11	12	30'	1.39	1.53	1.50	0.14	-	-	-		50
					0.33' - <2'	0.55	0.76	0.66	0.27	0.27	0.46	0.55	5	-
				4	2' - <3'	0.55	0.76	0.66	0.14	-	-	-	Note	62
				to	3' - <5'	0.54	0.72	0.65	0.14	-	-	-		62
11' x 8'	11	11	11	12	5' - 10'	0.73	0.79	0.82	0.14	-	-	-	ral	55
					15'	0.93	1.03	1.03	0.14	-	_	-	General	50
	11	11	11	8	20'	1.21	1.39	1.36	0.14	-	-	-		50
	12	12.5	11	to	25'	1.34	1.56	1.50	0.14	-	-	-	See	50
	13.5	13.5	11	12	30'	1.41	1.66	1.65	0.14	-	-	-) S	50
					0.33' - <2'	0.60	0.81	0.71	0.27	0.27	0.48	0.60		
				4	2' - <3'	0.60	0.81	0.71	0.15	-	-	-		75
				to	3' - <5'	0.61	0.77	0.70	0.14	-	-	-		69
11' × 10'	11	11	11	12	5' - 10'	0.80	0.88	0.93	0.14	-	-	-		62
	1.1	1.1	1.1	8	15'	1.01	1.13	1.15	0.14	-	_	-		55
	11	11	11	-1	20'	1.30	1.52	1.52	0.14	-	-	-		50
	12	12.5	11	to 12	25' 30'	1.42	1.70	1.65	0.14	-	-	-	-	50
	13.5	14	11	12		1.53	1.77	1.74	0.14	- 0.27	- 0 E 1	- 0.64	-	50
				4	0.33' - <2' 2' - <3'	0.64	0.83	0.74	0.27	0.27	0.51	0.64		- 06
						0.64	0.83	0.74	0.21					86
11' × 11'	11	11	11	to 12	3' - <5' 5' - 10'	0.67 0.88	0.79	0.75	0.21	_	_	-	-	75 69
11 X 11		'1	'1	12	3 - 10 15'	1.09	1.19	1.23	0.19	_				55
	11	11	11	8	20'	1.09	1.19	1.60	0.16	_	_	-		55
	12	12.5	11	to	25'	1.54	1.77	1.73	0.15	_	_	_	-	50
	13.5	12.5	11.5	12	30'	1.54	1.77	1.76	0.13	_	_	_		50
	15.5	14	11.5	1 12		1.57	1.//	1.70	0.14			_	L	1 50

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

DESCRIPTION:

FDOT

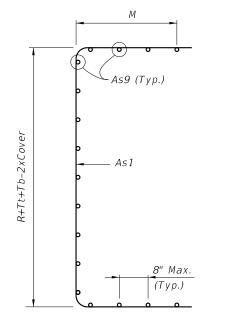
T	ABLE	16 - 5	STANI	DARD P	RECAST BO	X CU	LVERT						2' SP/	4 <i>NS</i>
SPAN x RISE (S) (R)	TOP	ВОТ.	Dor. SIDE MONCH		DESIGN EARTH COVER	REINFORCEMENT AREAS (sq. in./Ft.)							As1 EXT LENGTH	
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(M) (in.)
				4	0.33' - <2'	0.59	0.64	0.51	0.29	0.29	0.52	0.55		_
			12	to	2' - <3'	0.60	0.64	0.51	0.15	-	-	-		73
	12	12			3' - <5'	0.60	0.61	0.51	0.15	-	-	-		66
12' x 4'					5' - 10'	0.81	0.61	0.61	0.15	-	-	-		66
				12	15'	1.04	0.80	0.77	0.15	-	-	-		59
					20'	1.37	1.08	1.03	0.15	-	-	-		59
	13	13	12	8 to	25'	1.58	1.26	1.21	0.15	-	-	-		59
	14.5	14.5	12	12	30'	1.63	1.38	1.34	0.15	-	-	-		53
				4	0.33' - <2'	0.56	0.70	0.57	029	0.29	0.47	0.52		_
			12	to	2' - <3'	0.56	0.70	0.57	0.15	-	-	-		66
	12	12			3' - <5'	0.56	0.67	0.57	0.15	-	-	-		59
12' x 6'	'-	'-			5' - 10'	0.74	0.69	0.70	0.15	-	-	-		59
12 % 0				12	15'	0.94	0.90	0.88	0.15	-	-	-		53
				12	20'	1.23	1.22	1.17	0.15	-	-	-		53
	13	13	12	8 to	25'	1.40	1.42	1.37	0.15	-	-	-		53
	14.5	15	12	12	30'	1.44	1.54	1.48	0.15	-	-	-		53
	12	12	12	4 to 12	0.33' - <2'	0.55	0.75	0.63	0.29	0.29	0.45	0.53		-
					2' - <3'	0.55	0.75	0.63	0.15	_	-	-		66
					3' - <5'	0.55	0.73	0.63	0.15	-	-	-		59
12' x 8'					5' - 10'	0.73	0.77	0.79	0.15	-	-	-	'al	59
12 / 0					15'	0.93	1.00	0.99	0.15	-	-	-	General	53
	12	12	12	8	20'	1.21	1.35	1.31	0.15	-	-	-		53
	13	13.5	12	to	25'	1.35	1.55	1.48	0.15	_	-	-	See	53
	14.5	15	12	12	30'	1.40	1.67	1.62	0.15	-	-	-	S	53
		12			0.33' - <2'	0.57	0.80	0.68	0.29	0.29	0.46	0.57		_
				4	2' - <3'	0.57	0.80	0.68	0.15	-	-	_		73
	12		12	to	3' - <5'	0.59	0.77	0.68	0.15	-	-	-		66
12' x 10'	12	12	12	12	5' - 10'	0.78	0.85	0.89	0.15	-	-	-		59
12 × 10					15'	0.98	1.10	1.11	0.15	-	-	-		53
	12	12	12	8	20'	1.26	1.47	1.45	0.15	-	-	-		53
	13	13.5	12	to	25'	1.39	1.68	1.63	0.15	-	-	-		53
	14.5	15	12	12	30'	1.48	1.79	1.76	0.15	-	-	-		53
12' × 12'	12	12	12		0.33' - <2'	0.65	0.84	0.73	0.29	0.29	0.50	0.65		_
				4 to 12	2' - <3'	0.65	0.84	0.73	0.23	-	-	-		93
					3' - <5'	0.68	0.81	0.75	0.22	-	-	-		80
	'-				5' - 10'	0.90	0.94	1.01	0.21	-	-	-		73
12 / 12					15'	1.12	1.20	1.24	0.18	-	-	-		59
	12	12	12	8	20'	1.42	1.60	1.61	0.16	-	-	-		59
	13	13.5	12	to	25'	1.57	1.81	1.78	0.16	-	-	-		53
				1			1		1		1	1	1	

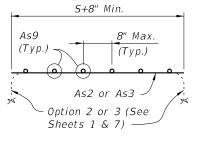
12

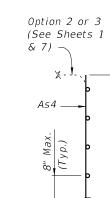
- 1. See Sheet 2 of 14 for General Notes.
- 2. See Sheet 7 of 14 for Reinforcing Details and dimension locations.

1.63 1.86 1.85 0.15

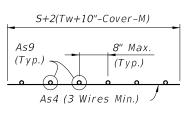
WELDED WIRE REINFORCEMENT BENDING DIAGRAM

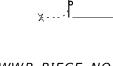






WWR PIECE NO. 2 (2 Reqd. per segment)



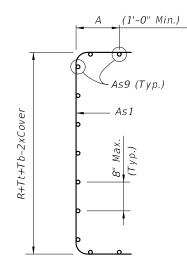


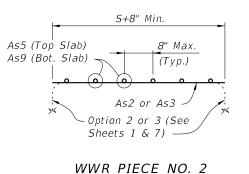
WWR PIECE NO. 1 (2 Regd. per segment)

WWR PIECE NO. 4 (Tongue Reinforcement) (4 Regd. per segment)

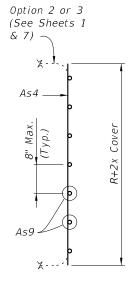
WWR PIECE NO. 3 (2 Regd. per segment)

TYPE 2 BOX SECTION (DESIGN EARTH COVER 2' OR GREATER)





(2 Reqd. per segment) S+2(Tw+10"-Cover-A) 8" Max. (Typ.)As7 (Top Slab) As8 (Bot. Slab)



WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (2 Regd. per segment)

WWR PIECE NO. 3 (2 Regd. per segment)

TYPE 1 BOX SECTION (DESIGN EARTH COVER LESS THAN 2')

REINFORCEMENT NOTES:

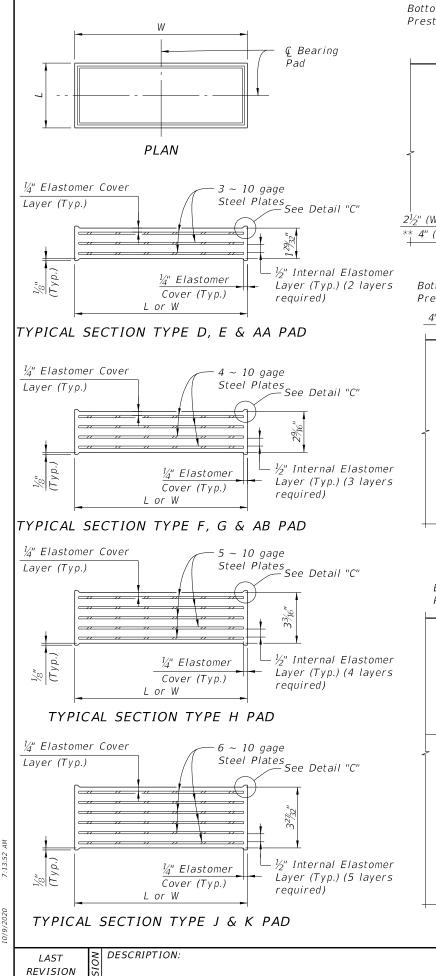
- 1. Reinforcement bending dimensions are out-to-out.
- 2. See General Notes 4, 5 and 6 on Sheet 2.
- 3. See Tables 1 thru 16 for dimensions M, R, S, Tb, Tt and Tw.
- 4. Dimension "A" is determined by the Manufacturer in accordance with the requirements of Detail "B" on Sheets 1 and 7.

14.5 15 12.5

DESCRIPTION: 07/01/13

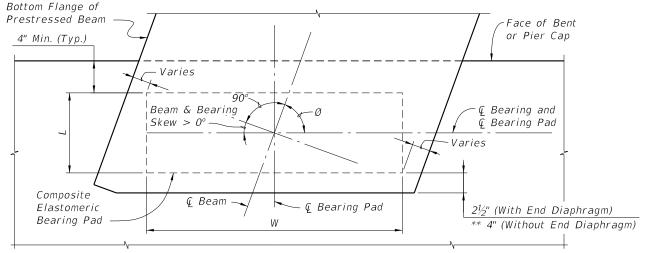


53

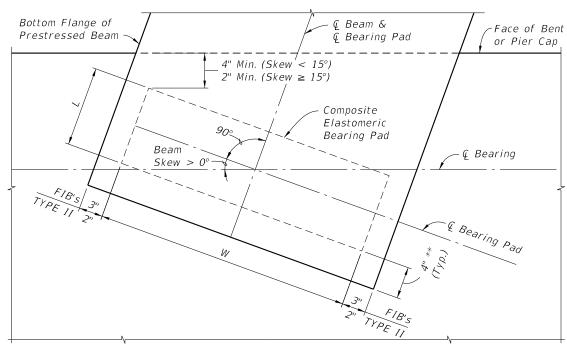


Bottom Flange of Prestressed Beam ———		
4" Min. FIB's	3" W	3" FIB'S Face of Ben 2" TYPE II or Pier Cap
Composite Elastomeric	90°	© Bearing and © Bearing Pad
Bearing Pad	C Poam S C P	Bearing Pad

PARTIAL PLAN (Beam & Bearing Skew = 0°)



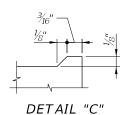
PARTIAL PLAN (Beam & Bearing Skew > 0°) (Use Index 450-511)



PARTIAL PLAN WITH SQUARED END BEAM (Use Index 450-512) (Beam Skew > 0° ; Bearing Skew = 0°)

	BEAM TYPE	BEARII DIMEN	NG PAD ISIONS	*BEVELED BEARING PLATE DIMENSIONS			
PAD TYPE (See Note 1)		L	W	С	D		
D (G=110psi)		8"	2'-8"	1'-0"	3'-0"		
E (G=110psi)		10"	2'-8"	1'-0"	3'-0"		
F (G=110psi)	I-BEAM	10"	2'-8"	1'-0"	3'-0"		
G (G=150psi)		10"	2'-8"	1'-0"	3'-0"		
H (G=150psi)	FLORIDA	10"	2'-8"	1'-0"	3'-0"		
(G=150psi)		10"	2'-8"	1'-0"	3'-0"		
(G=150psi)		1'-0"	2'-8"	1'-1½"	3'-0"		
AA (G=110psi)	AASHTO TYPE II	10"	1'-2"	1'-0"	1'-4"		
AB (G=150psi)	AAS TYP	10"	1'-2"	1'-0"	1'-4''		

- * Work this sheet with the appropriate type Bearing Plate Detail (See Bearing Plate Data Table) and BEARING PAD DATA TABLE in the Structures Plans. See TABLE OF BEAM VARIABLES and BEARING PLATE DATA TABLE in the Structures Plans for locations where beveled bearing plates are required.
- ** Offset to End of Beam is reduced to 2" for Type K

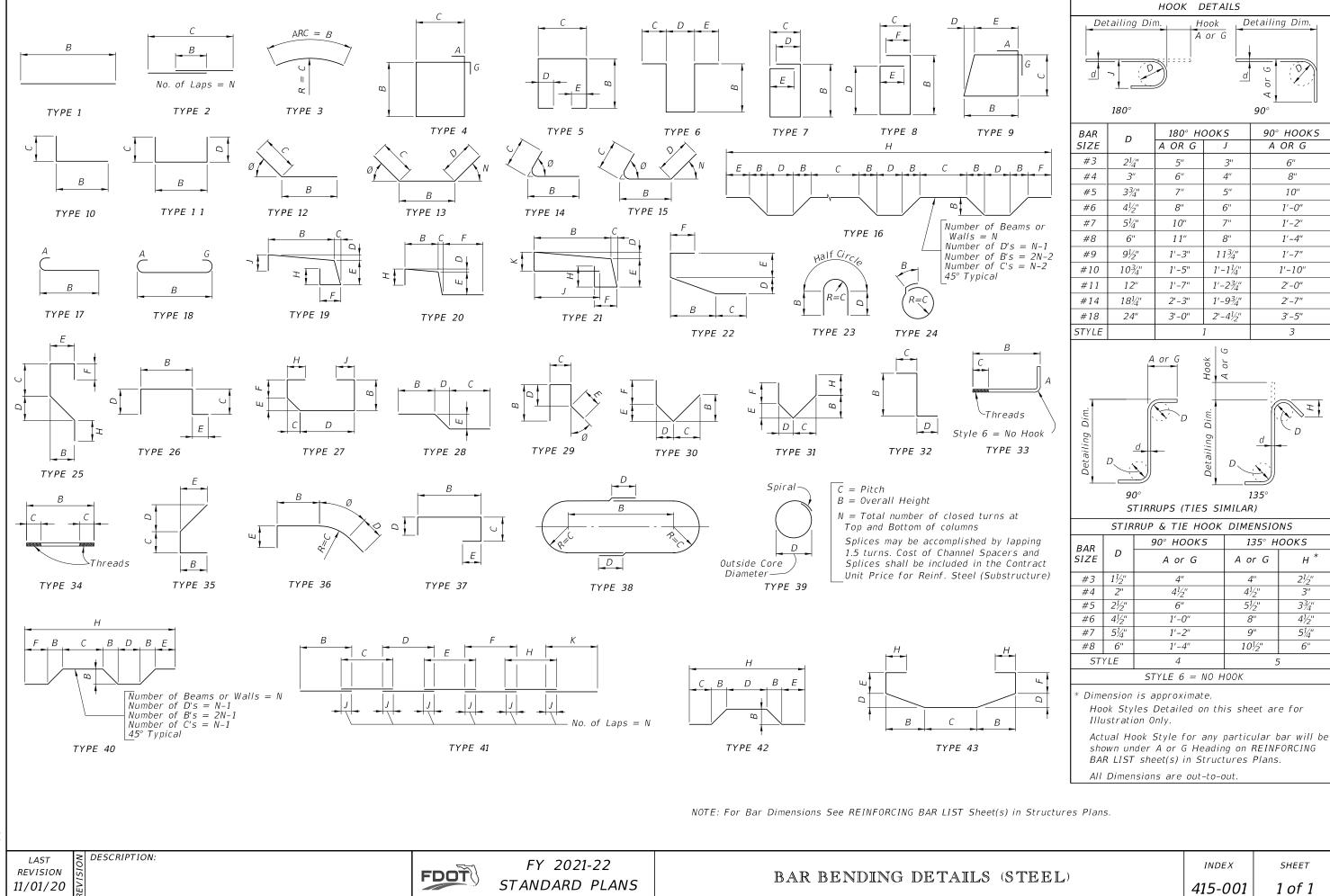


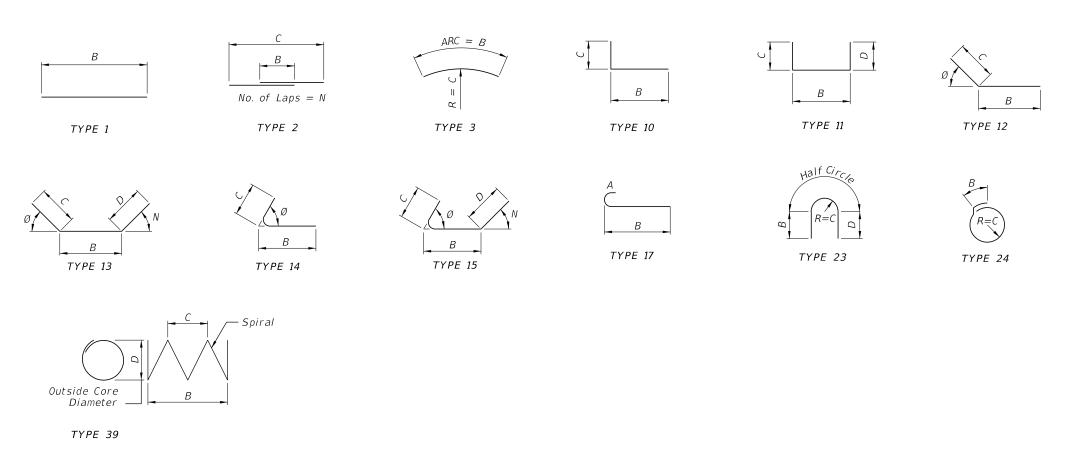
BEARING PAD NOTES:

- 1. Neoprene in Type D, E, F & AA bearing pads shall have a shear modulus (G) of 110 psi. Neoprene in Type G, H, J, K & AB bearing pads shall have a shear modulus (G) of 150 psi.
- 2. Steel Plates in bearing pads shall conform to ASTM A1011 Grade 36, Type 1.
- 3. See Bearing Pad Data Table in Structures Plans for quantities of Type D, E, F, G, H, J, K, AA and/or AB Bearing Pads.

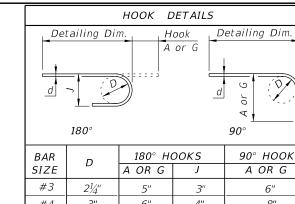
400-510 1 of 1

07/01/15





🗆 SINGLE BAR BENDING DETAILS 💳



D	180° H	OOKS	90° HOOKS			
D	A OR G	J	A OR G			
21/4"	5"	3"	6"			
3"	6"	4"	8"			
3¾"	7"	5"	10"			
4½"	8"	6"	1'-0"			
5½"	10"	7"	1'-2"			
6"	11"	8"	1'-4"			
	3" 3 ³ / ₄ " 4 ¹ / ₂ " 5 ¹ / ₄ "	D A OR G 2½" 5" 3" 6" 3¾" 7" 4½" 8" 5½" 10"	A OR G J 2½" 5" 3" 3" 6" 4" 3¾" 7" 5" 4½" 8" 6" 5½" 10" 7"			

NOTES

GENERAL

All dimensions are out-to-out.

For Bar dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

SPIRALS (TYPE 39 BARS)

C = Pitch

B = Overall Height

N = Total number of closed turns at

Top and Bottom of columns

Spirals = 1.5 turns

Include spiral splice in Contract Unit Price for FRP Reinforcing.

H00K5

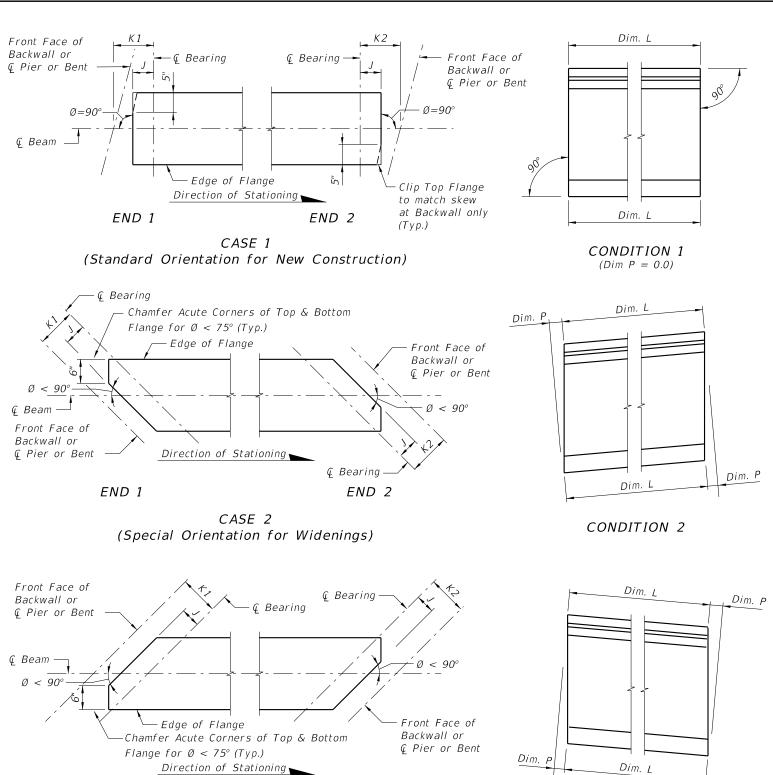
All Dimensions are approximate.

Hook Styles Detailed on this sheet are for Illustration Only. Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.

REVISION 11/01/20

DESCRIPTION:





END 2

CASE 3

(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

BEAM NOTES

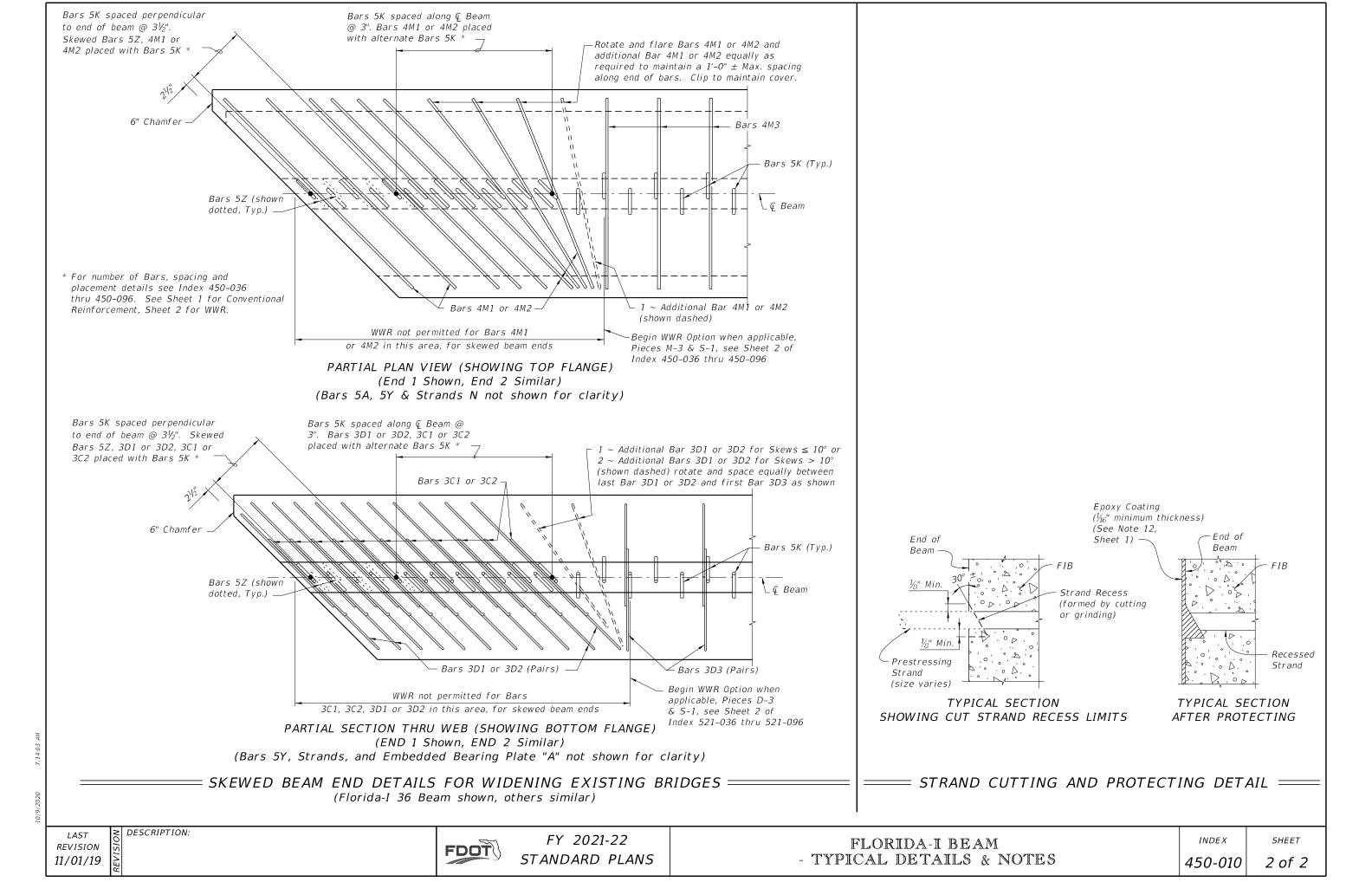
- 1. Work this Index with the Florida-I Beam Standard Details (Index 450-036 thru 450-096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum.
- 4. Strands N: ¾" Ø minimum, stressed to 10,000 lbs. each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
 - A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands.
 - B. For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
 - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
 - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
 - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.
- D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement.
- 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
 - B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
 - A. The superstructure environmental classification is slightly or moderately aggressive
 - B. Clear cover to adjacent steel reinforcing is 1"or greater
 - C. Hole inside diameter is 2" maximum
 - D. Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

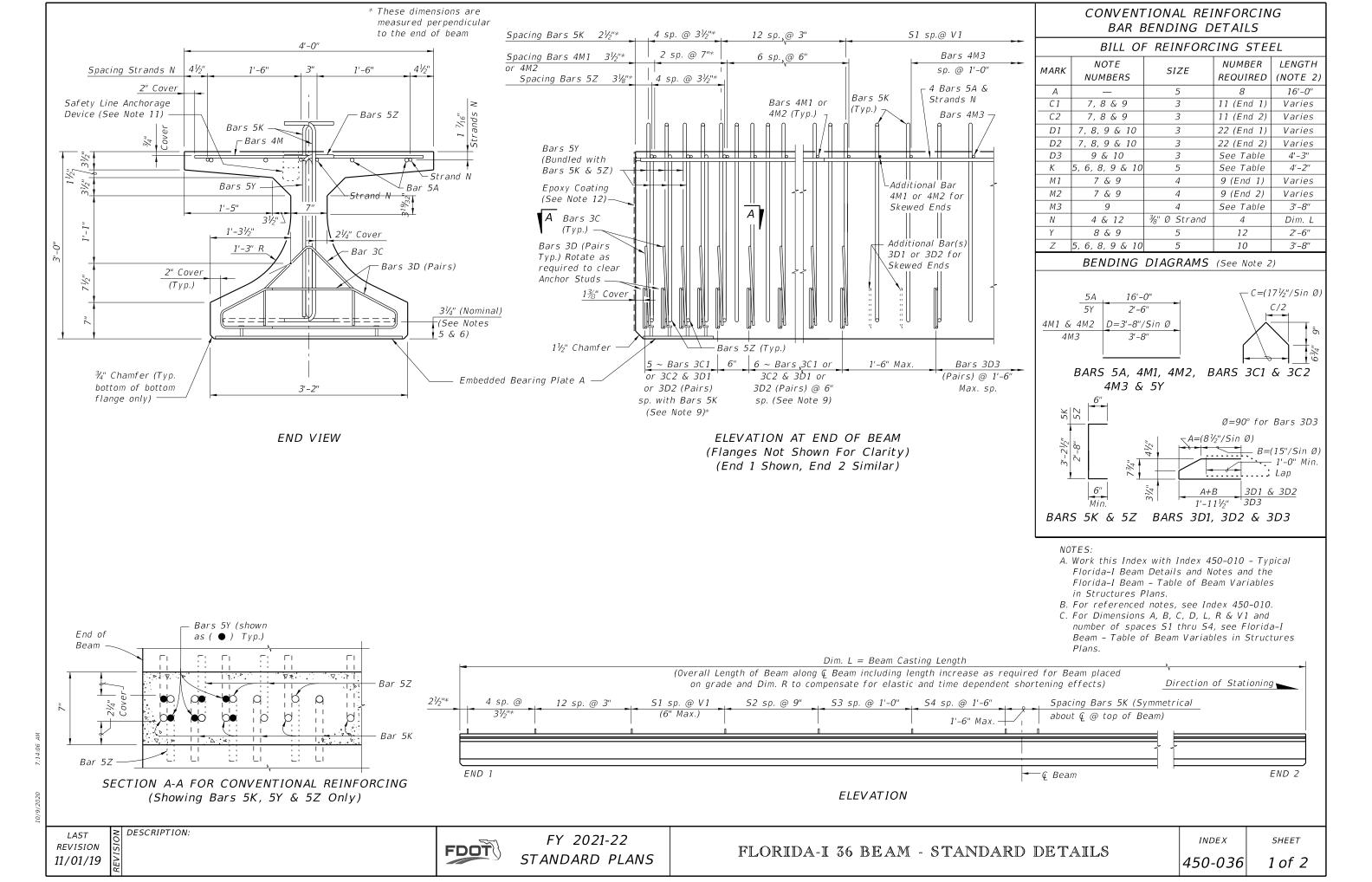
END 1

DESCRIPTION:

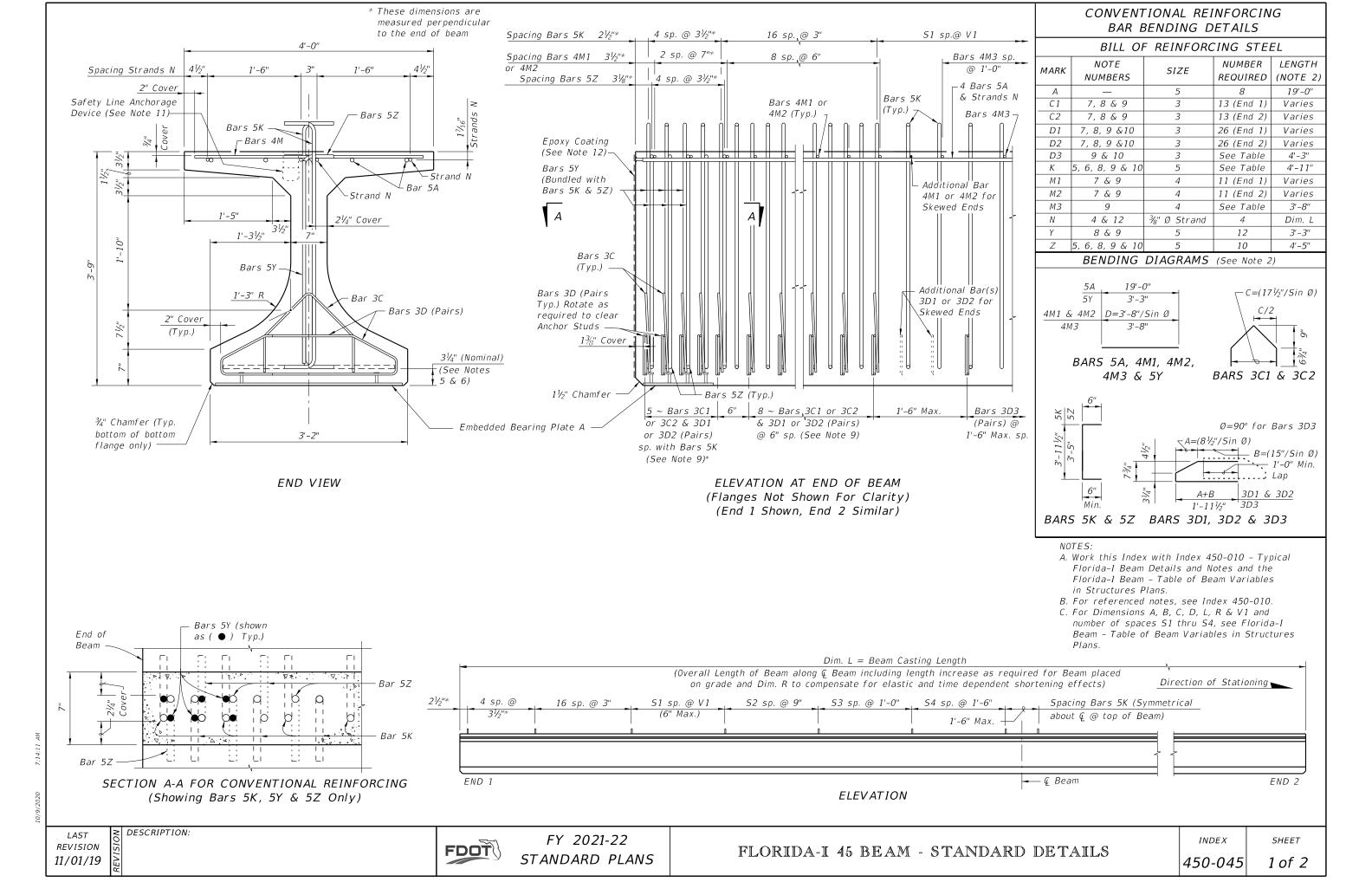
CONDITION 3

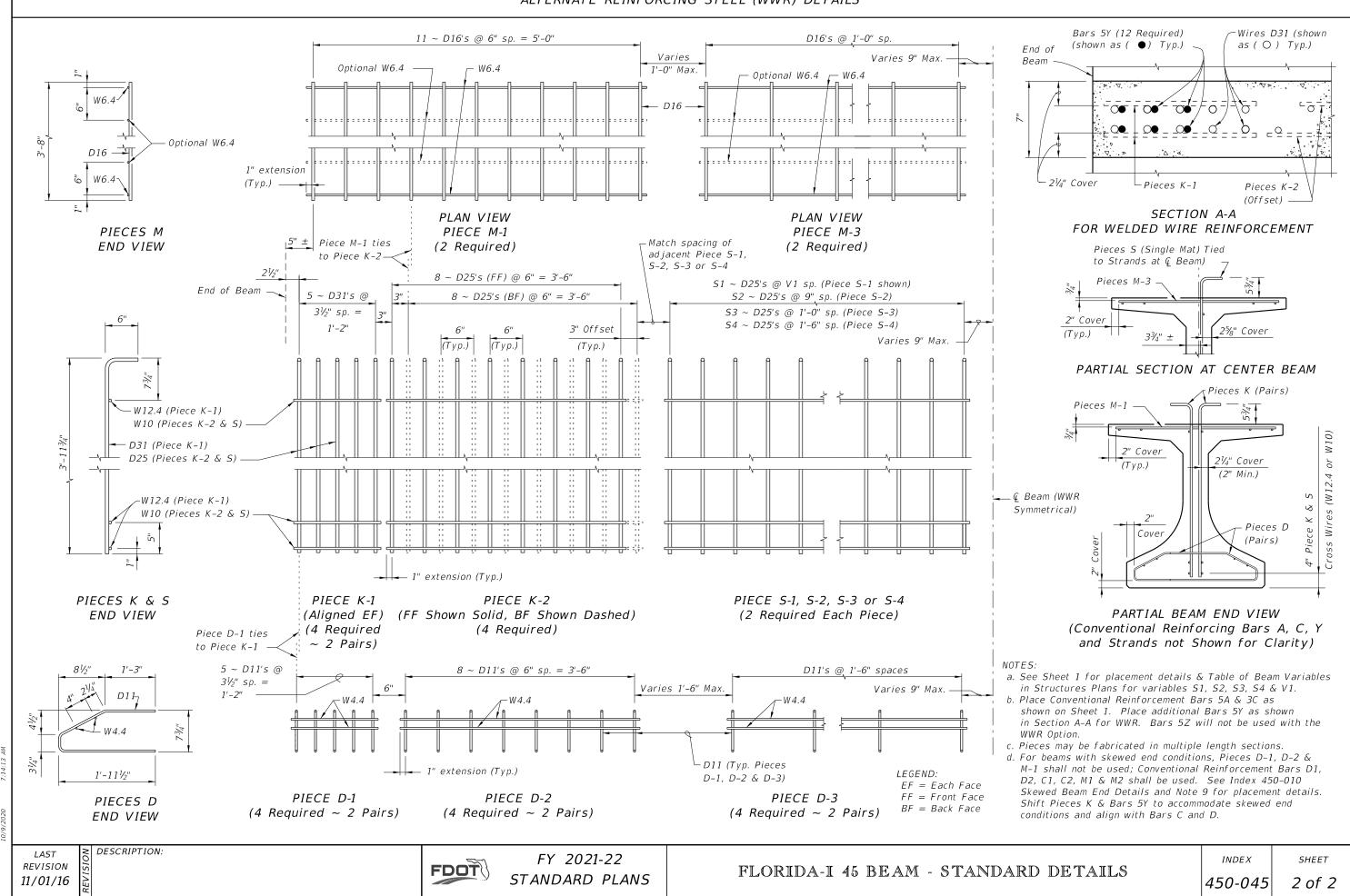
SCHEMATIC END ELEVATIONS OF BEAMS
(Showing Vertical Bevel of Beam End)

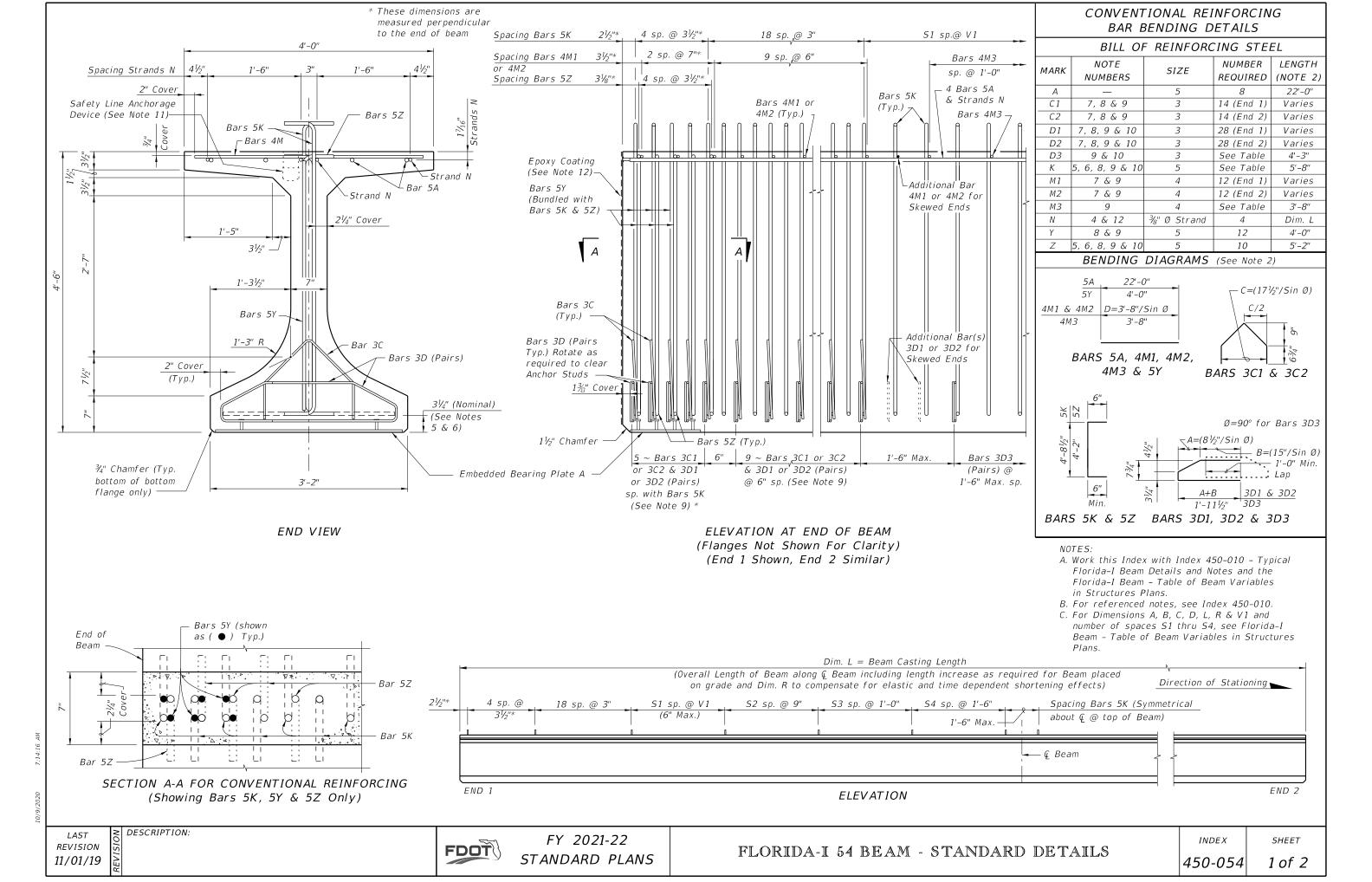


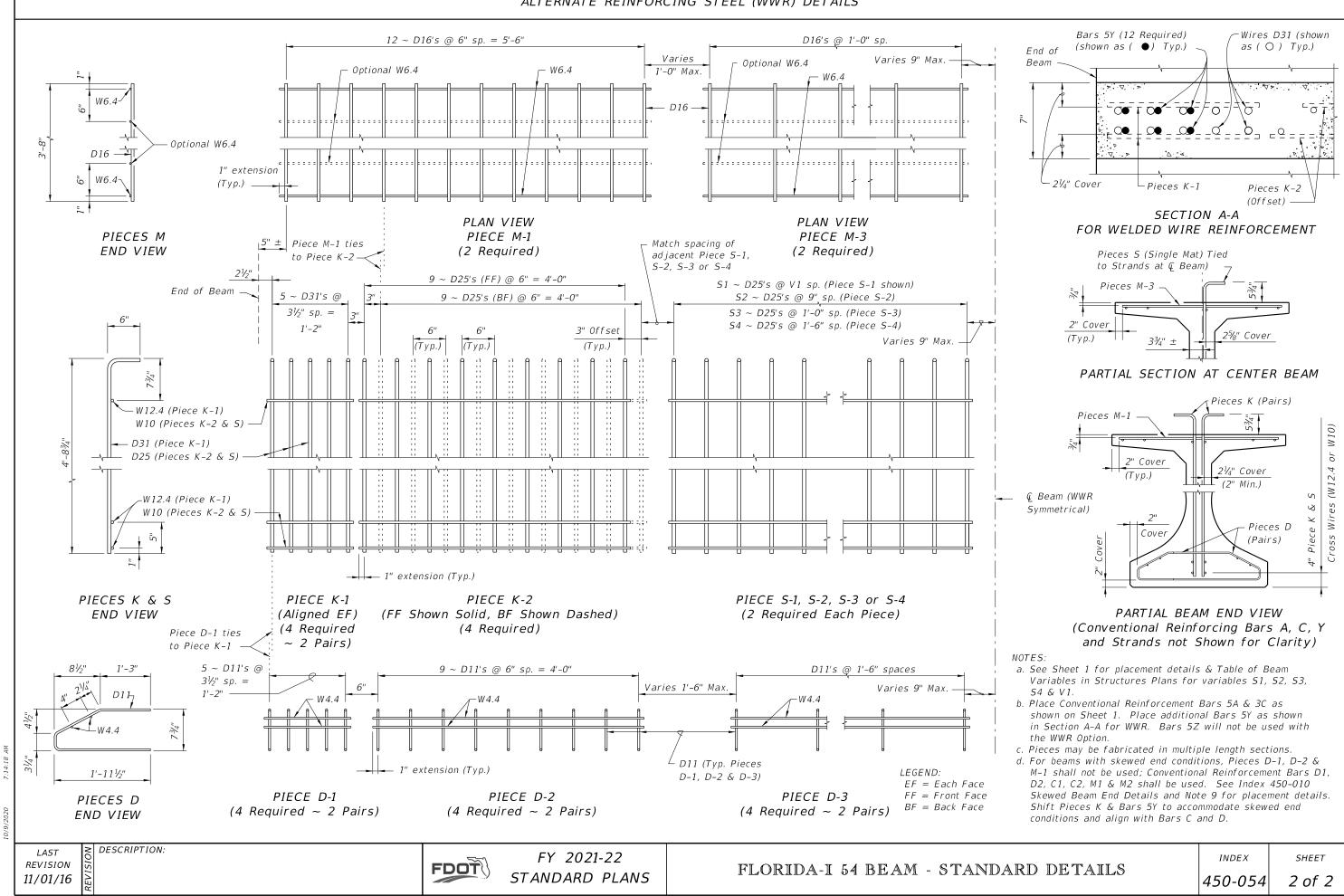


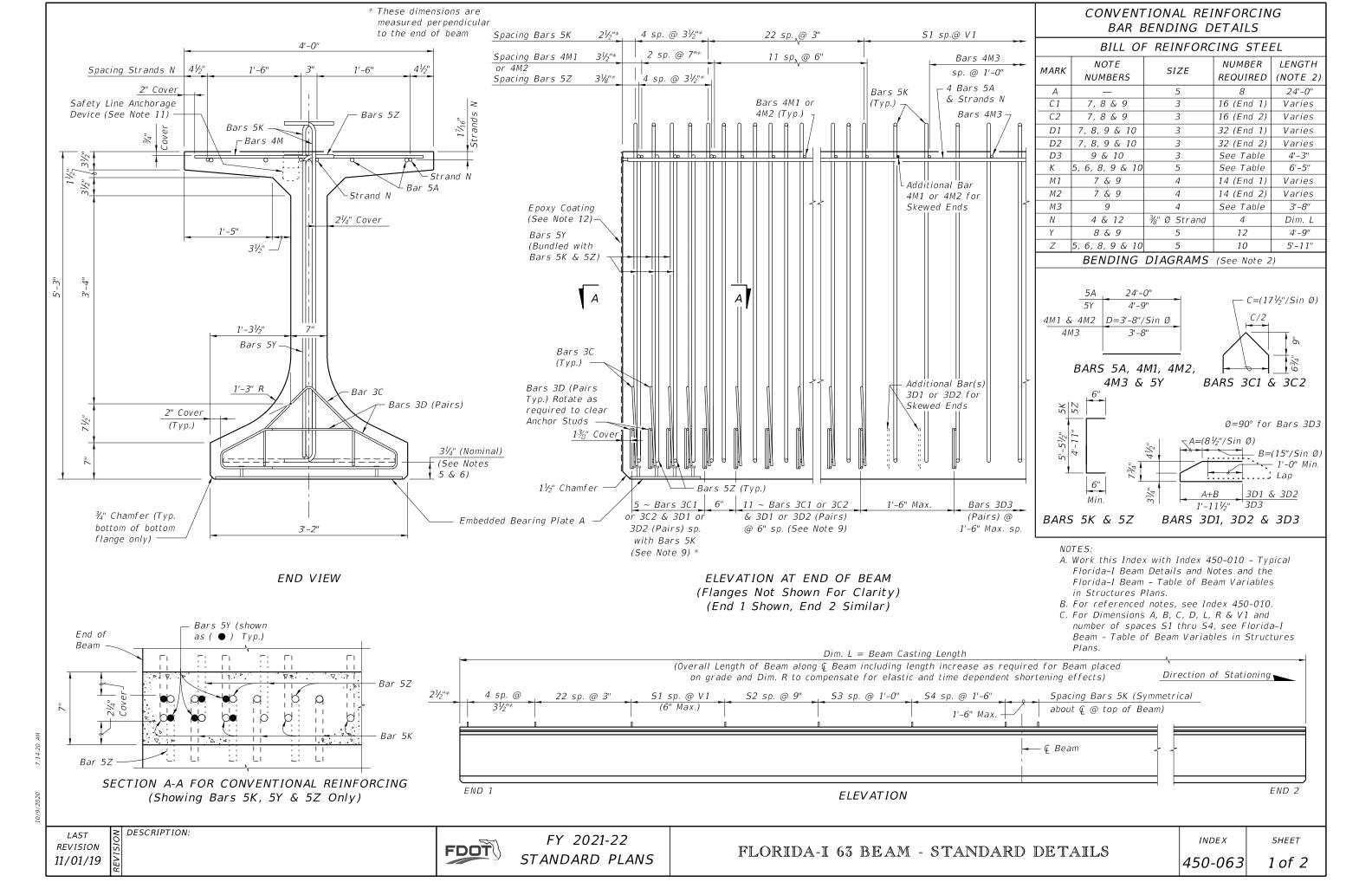
ALTERNATE REINFORCING STEEL (WWR) DETAILS 9 ~ D16's @ 6" sp. = 4'-0" D16's @ 1'-0" sp. Bars 5Y (12 Required) -Wires D31 (shown (shown as (●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max Optional W6.4 -W6.4 1'-0" Max. Beam — Optional W6.4 - W6.4 W6.4-D16-Optional W6.4 D16 1" extension W6.4 (Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) PLAN VIEW PLAN VIEW SECTION A-A PIECES M PIECE M-1 PIECE M-3 – Match spacing FOR WELDED WIRE REINFORCEMENT 5" ± Piece M-1 ties END VIEW of adjacent (2 Required) (2 Required) to Piece K-2 Piece S-1, S-2, Pieces S (Single Mat) Tied 6 ~ D25's (FF) S-3 or S-4 21/5" to Strands at © Beam) S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam — Pieces M-3 6 ~ D25's (BF) S2 ~ D25's @ 9" sp. (Piece S-2) @ 6'' = 2'-6'' $3\frac{1}{2}$ " sp. = S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 2" Cover Varies 9" Max. 25/8" Cover (Typ.)(Typ.) (Typ.)(Typ.)3¾" ± PARTIAL SECTION AT CENTER BEAM W12.4 (Piece K-1) Pieces K (Pairs) W10 (Pieces K-2 & S) Pieces M-1 W10) or D31 (Piece K-1 (W12.4 2" Cover D25 (Pieces K-2 & S) -21/4" Cover (Typ.)(2" Min.) Ø Wires (.__Ç Beam (WWR -W12.4 (Piece K-1) Symmetrical) Pieces D W10 (Pieces K-2 & S) Piece Cover (Pairs) 1" extension (Typ.) PARTIAL BEAM END VIEW PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 (Conventional Reinforcing Bars A, C, Y END VIEW (Aligned EF) (FF Shown Solid, (2 Required Each Piece) and Strands not Shown for Clarity) (4 Required ~ BF Shown Dashed) Piece D-1 ties to Piece K-1 2 Pairs) (4 Required) NOTES: a. See Sheet 1 for placement details & Table of Beam 5 ~ D11's @ $6 \sim D11's @ 6'' sp. = 2'-6''$ D11's @ 1'-6" spaces 1'-3" Variables in Structures Plans for variables S1, S2, S3, $3\frac{1}{2}$ " sp. = Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, - 1" extension (Typ.) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index 450-010 D-1, D-2 & D-3) EF = Each FaceSkewed Beam End Details and Note 9 for placement details. PIECE D-1 PIECE D-2 PIECE D-3 FF = Front Face Shift Pieces K & Bars 5Y to accommodate skewed end PIECES D BF = Back Faceconditions and align with Bars C and D. (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) **END VIEW** DESCRIPTION: FY 2021-22 INDEX SHEET REVISION FDOT FLORIDA-I 36 BEAM - STANDARD DETAILS STANDARD PLANS 11/01/16 450-036 2 of 2

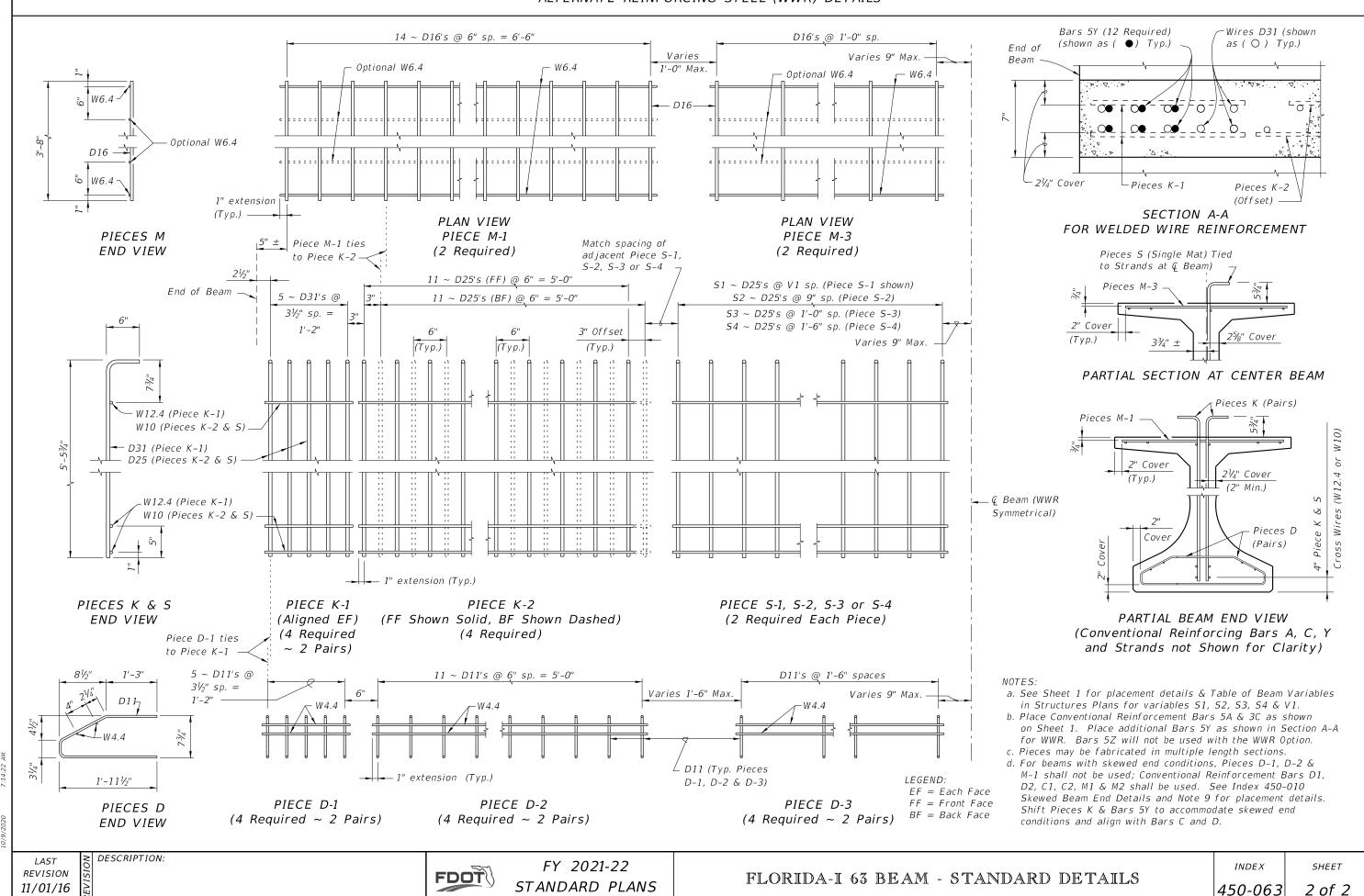


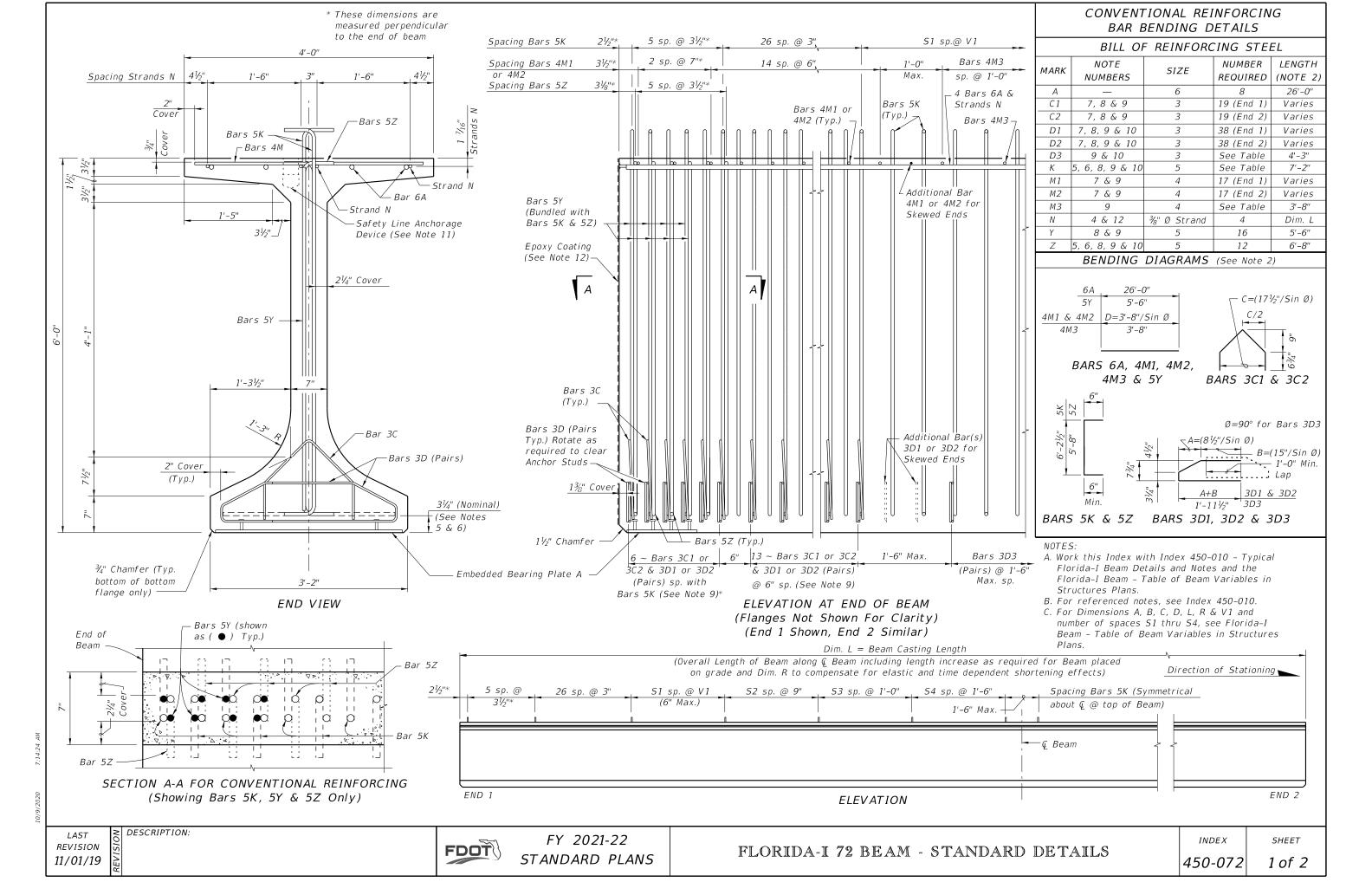


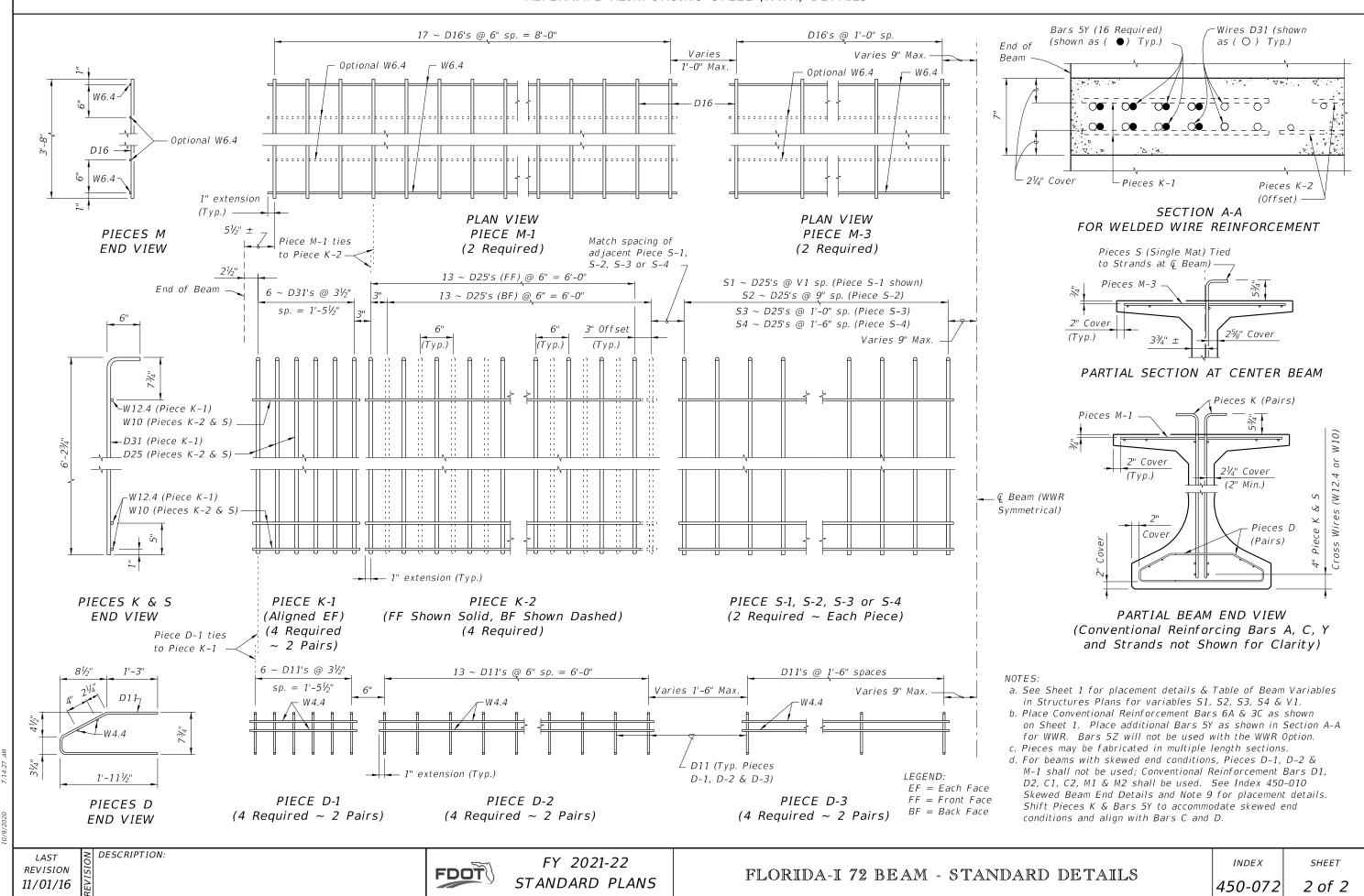


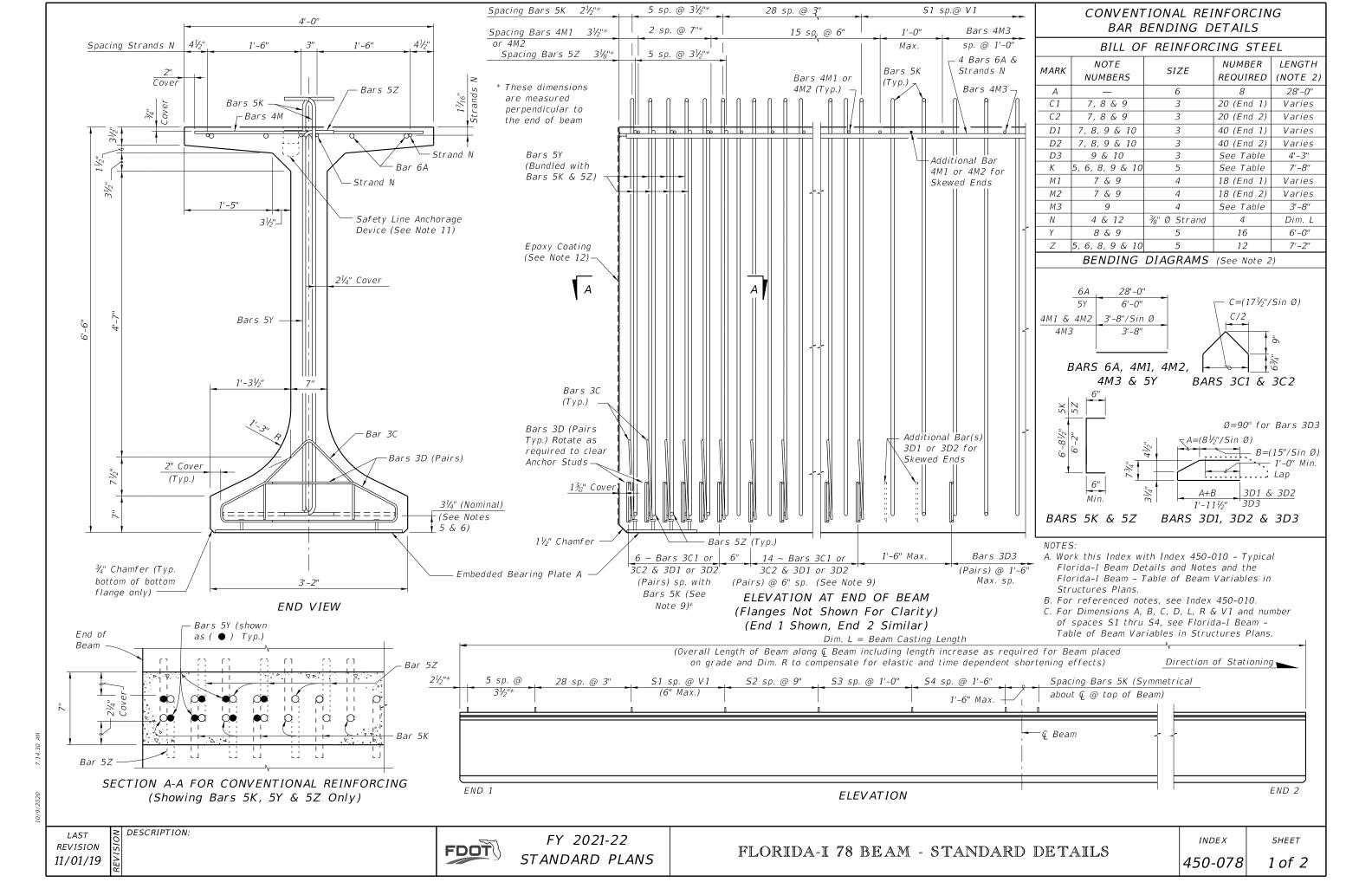


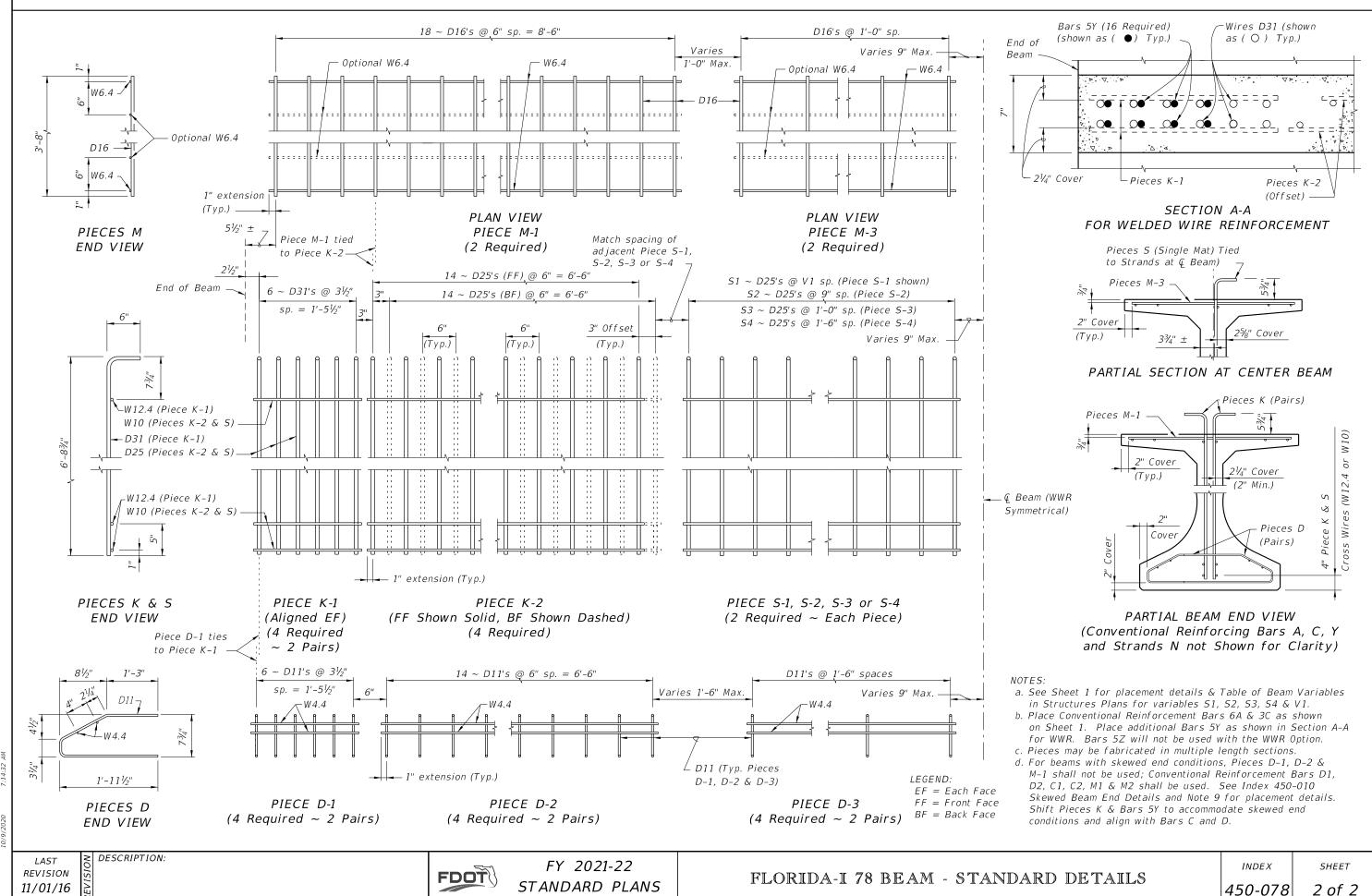




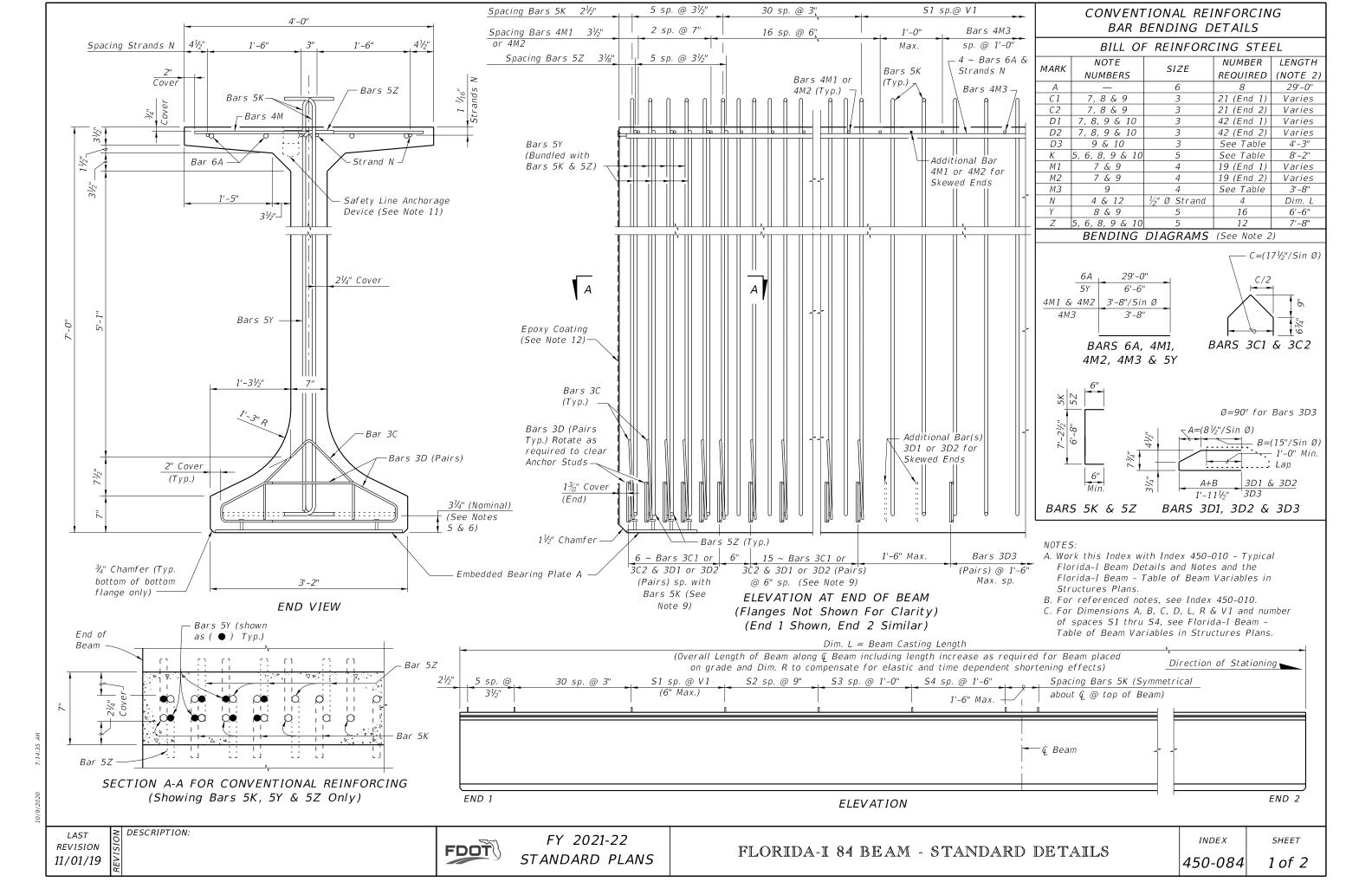


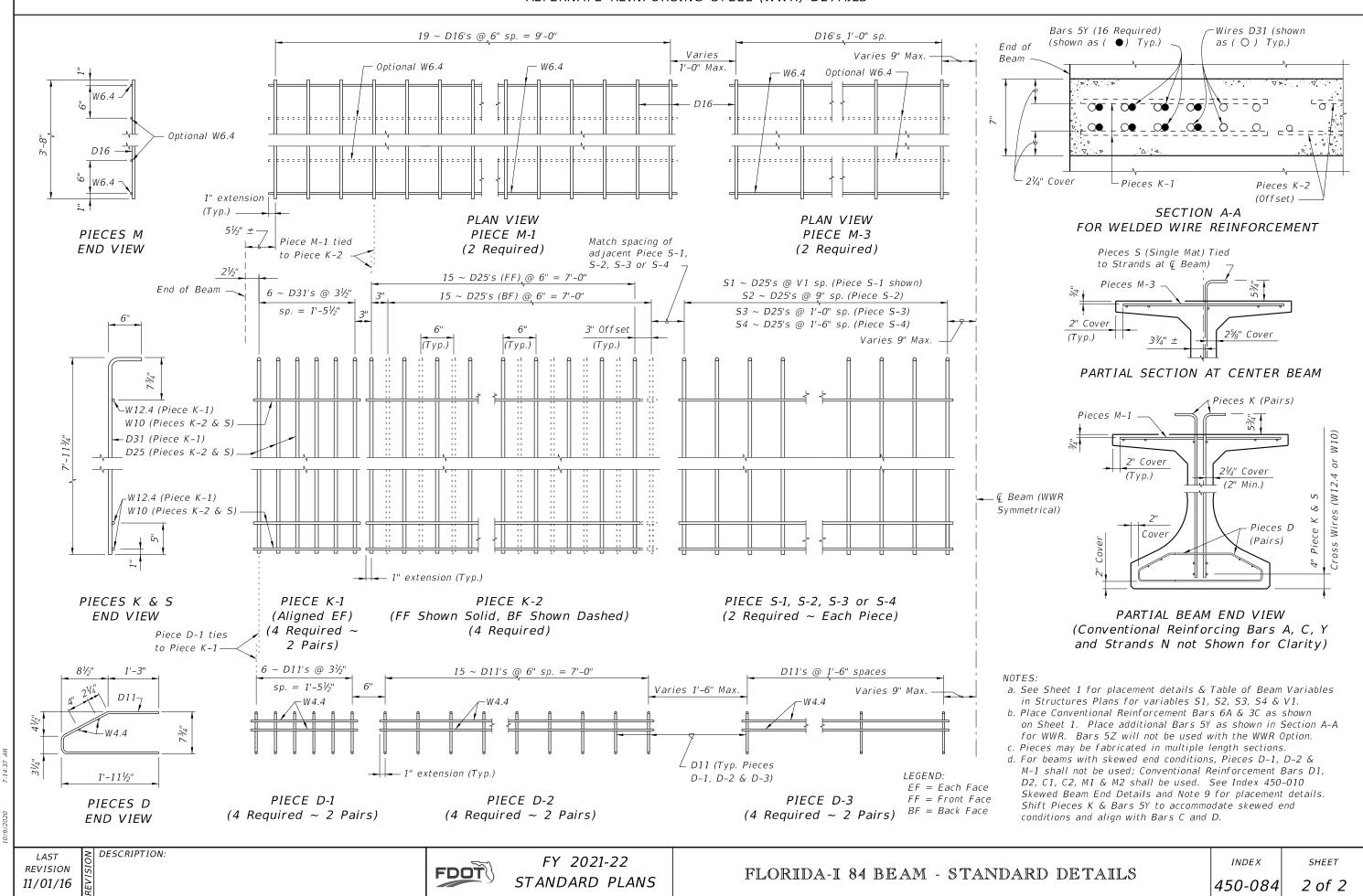


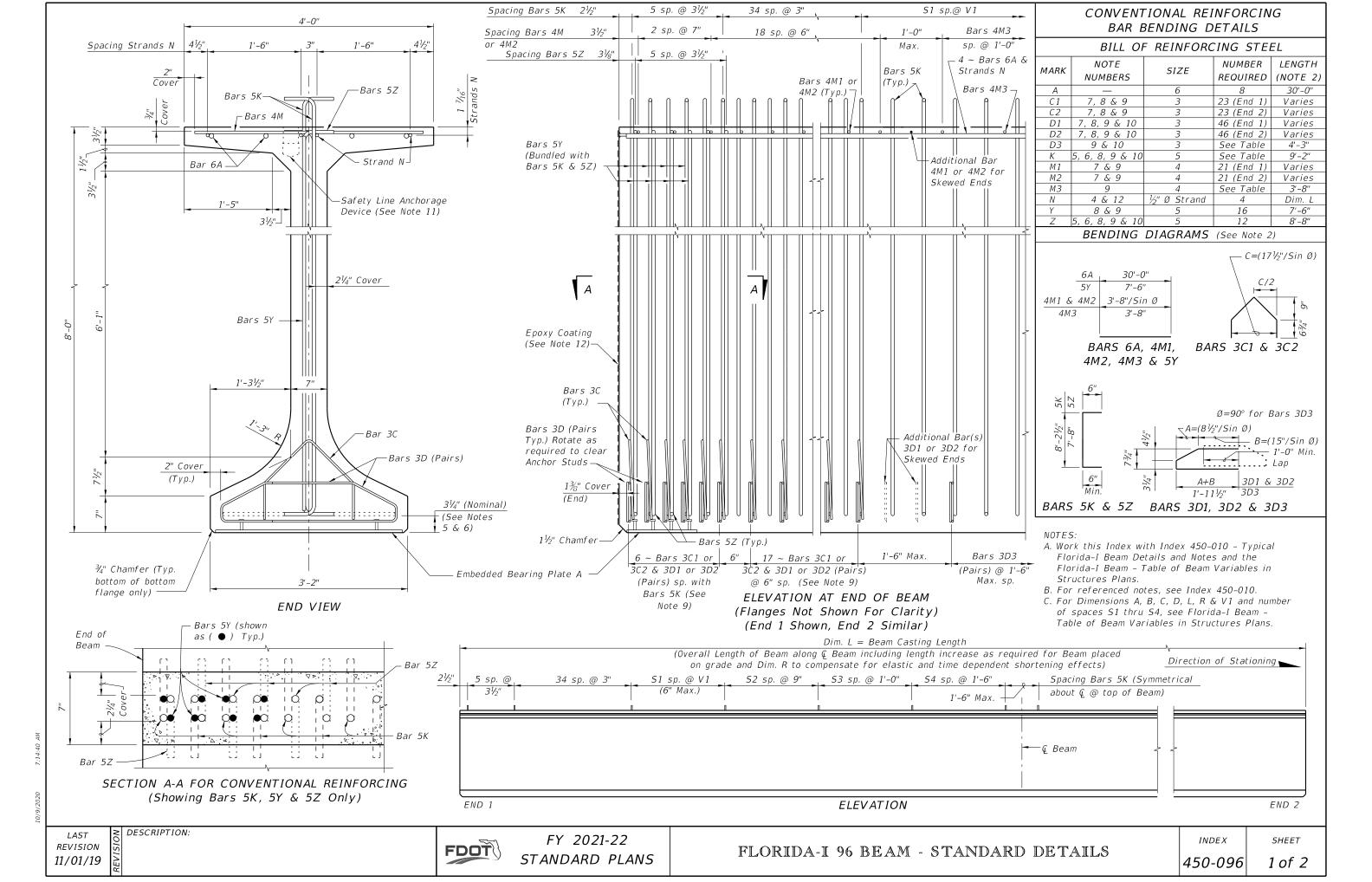


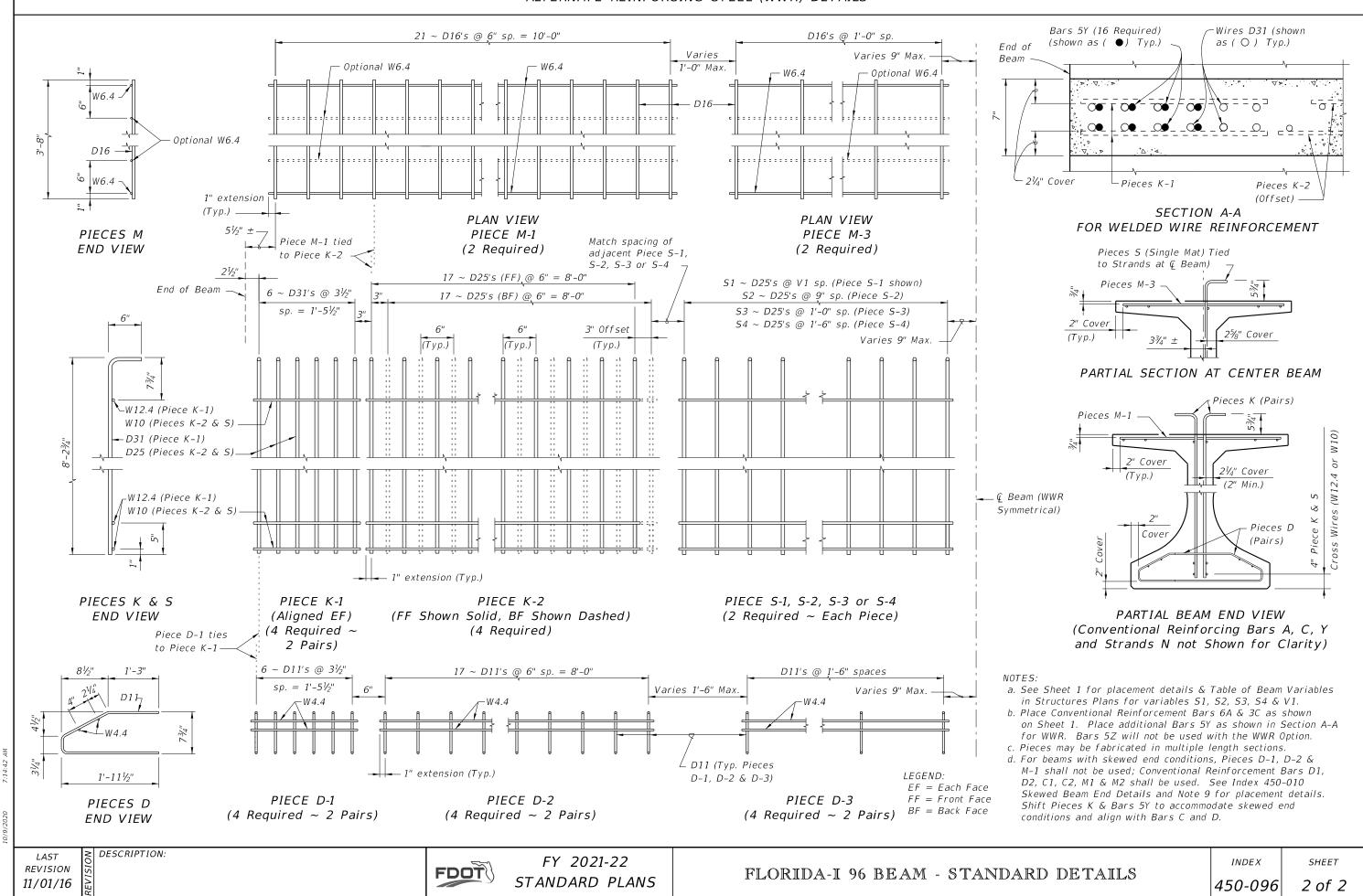


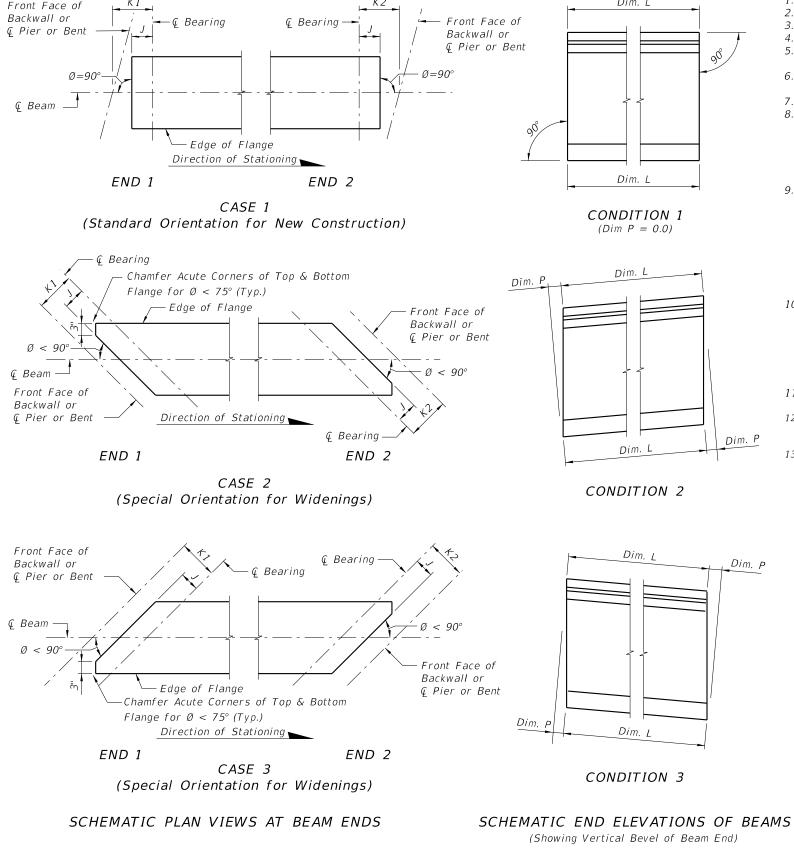
10/9/2020











BEAM NOTES

- 1. Work this Index with the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out to out.
- 3. Concrete cover: 2 inches minimum.
- 4. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 5. Place one (1) Bar 4K or 5Z at each location. Alternate the direction of the ends for each
- 6. Tie Bars 4K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. Place Bars 3D1 in beam END 1, and Bars 3D2 in beam END 2.
- 8. For Beams with vertically beveled end conditions:
 - A. Place first row of Bars 3D1, 3D2, 4K, 4Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1".
 - For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
- 9. For beams with skewed end conditions:
 - WWR is not permitted for end reinforcement Bars 3D1, and 3D2 on skewed ends; use bar reinforcement.
 - Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3D1, 3D2, 4K, 4Y and 5Z placed within the limits of the spacing for Bars 3D in "ELEVATION AT END OF BEAM".
 - Beyond the limits of the spacing for Bars 3D, place Bars 4K perpendicular to the longitudinal axis of the beam. For placement see "SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES" (Sheet 2).
- 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 4K, and 5Z as shown on Sheet 4; except at skewed ends (See Note 9).
 - Bars 3D1 and 3D2 may be fabricated as a two-piece bar with a 1'-0" minimum lap splice of the bottom legs.
- For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands or above Strands N.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of required anchorage devices.
- 12. For beams with ends that will not to be encased in concrete diaphragms, cut wedges and recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See "STRAND
- CUTTING AND PROTECTING DETAIL" on Sheet 2. 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
- The superstructure environmental classification is slightly or moderately aggressive
- Clear cover to adjacent steel reinforcing is 1"or greater
- Hole inside diameter is 2" maximum
- Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

DETAILS AND NOTES

REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2021-22 STANDARD PLANS

Dim. L

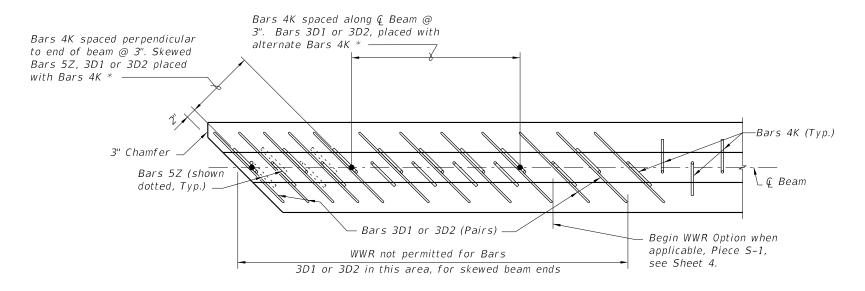
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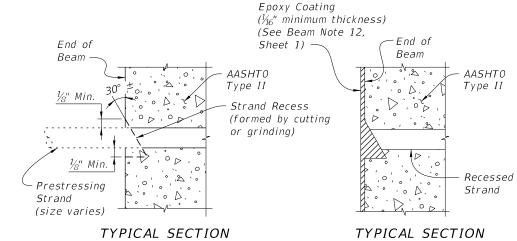
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for WWR.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE) (End 1 Shown, End 2 Similar) (Bars 4Y & Strands not shown for clarity)

= SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



SHOWING CUT STRAND RECESS LIMITS

AFTER PROTECTING

=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

REVISION 11/01/19

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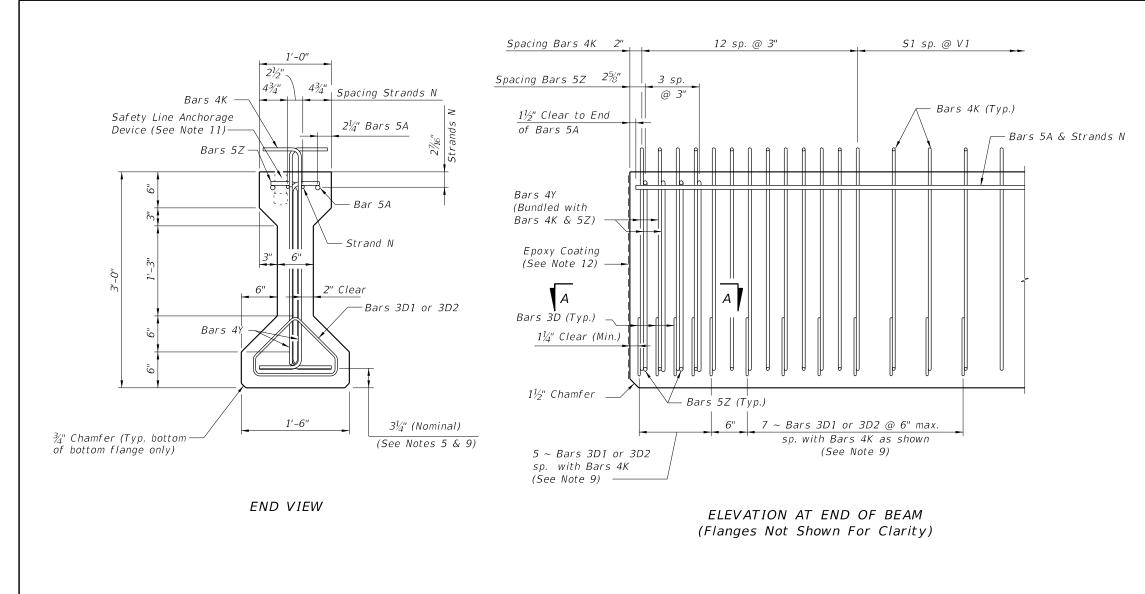
FY 2021-22 STANDARD PLANS

AASHTO TYPE II BEAM

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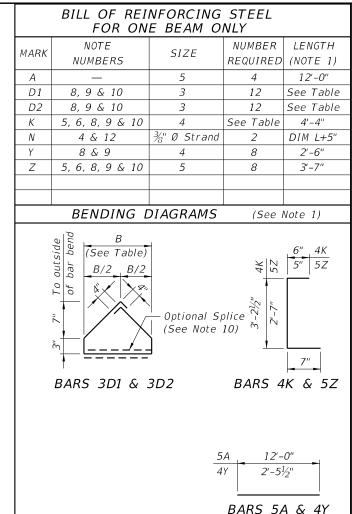
450-120



DESCRIPTION:

REVISION

11/01/19



NOTES

AASHTO TYPE II BEAM

Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.

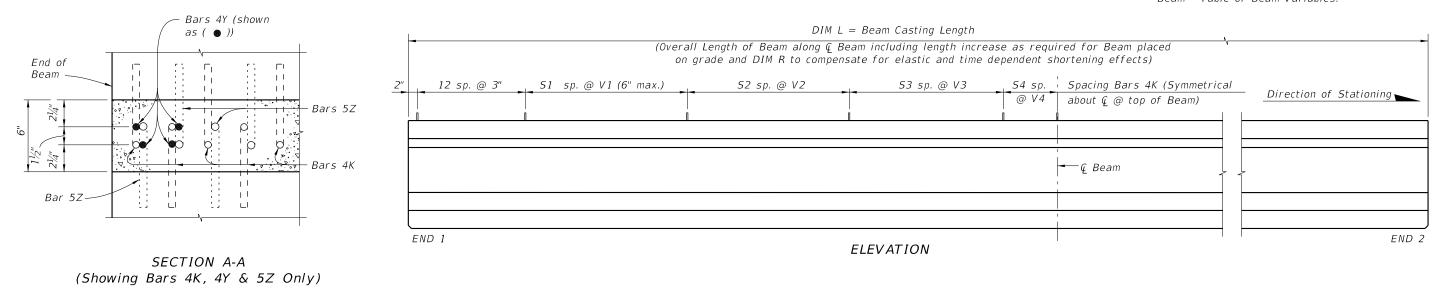
STANDARD DETAILS

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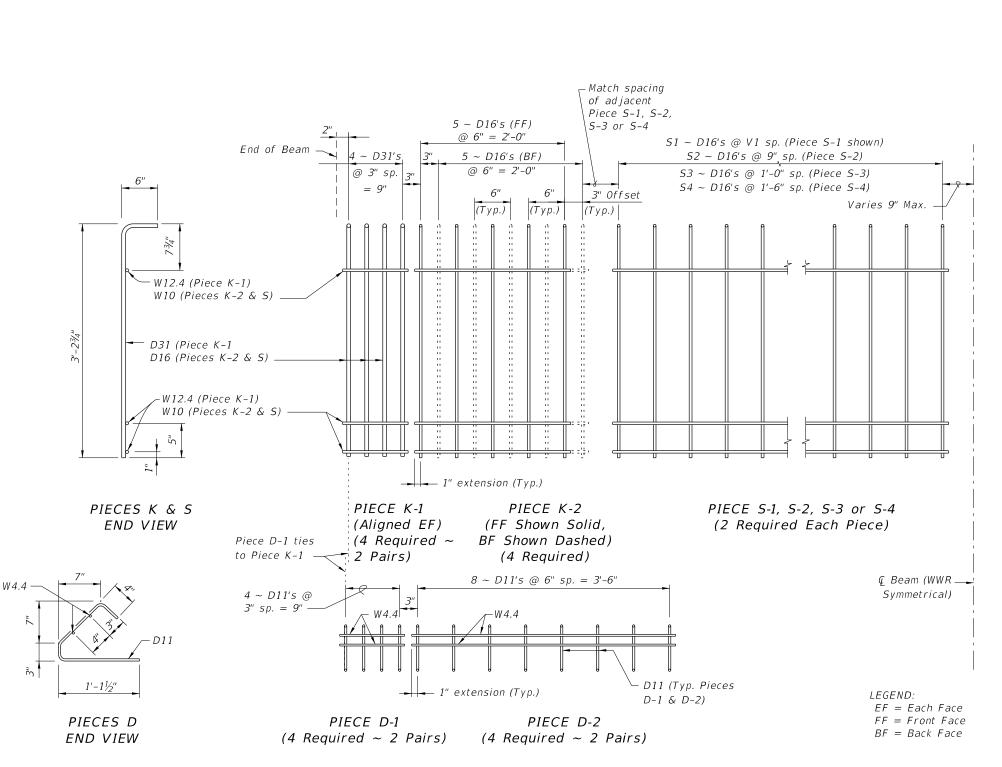
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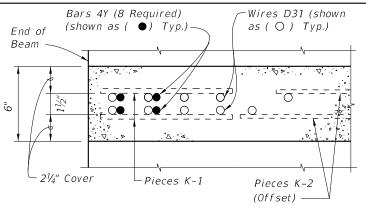


FY 2021-22

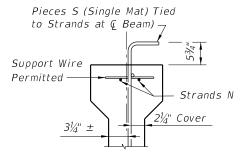
STANDARD PLANS

FDOT

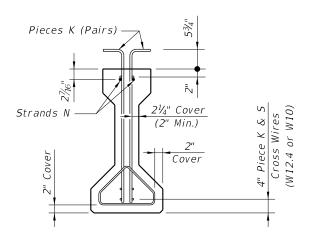




SECTION A-A FOR WELDED WIRE REINFORCEMENT



PARTIAL SECTION AT CENTER BEAM



PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, 54 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

STANDARD DETAILS

REVISION 11/01/16

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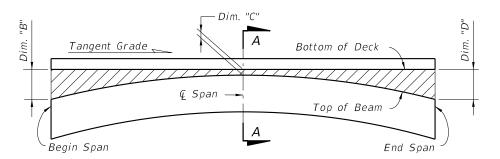
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FY 2021-22 STANDARD PLANS

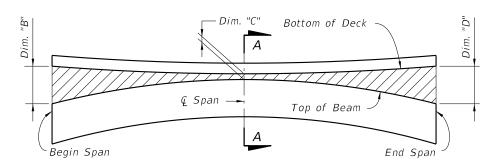
AASHTO TYPE II BEAM

INDEX SHEET

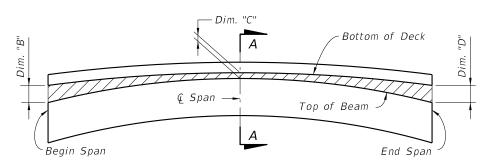
450-120 4 of 4



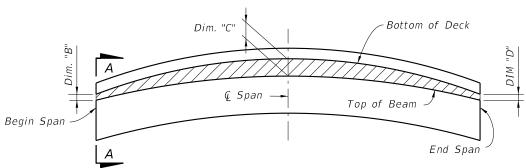
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG Q BEAM) (CASE 3)

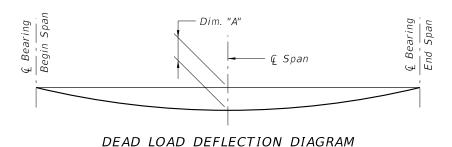


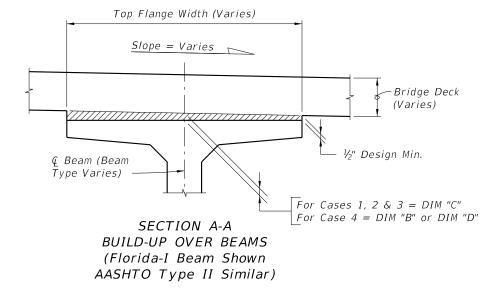
BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG Q BEAM) (CASE 4)

BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.

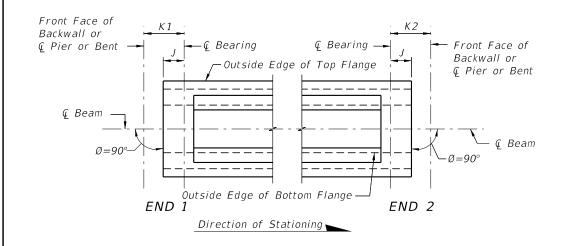




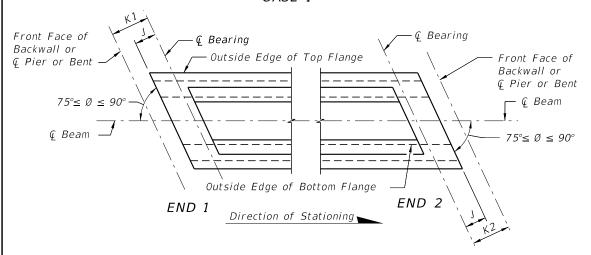
* NOTE:

Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.

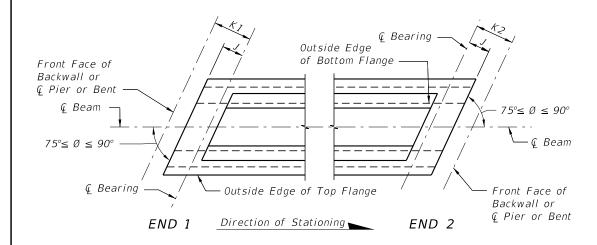
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CASE 1



CASE 2



CASE 3

SCHEMATIC PLAN VIEWS AT BEAM ENDS =

REVISION 11/01/16

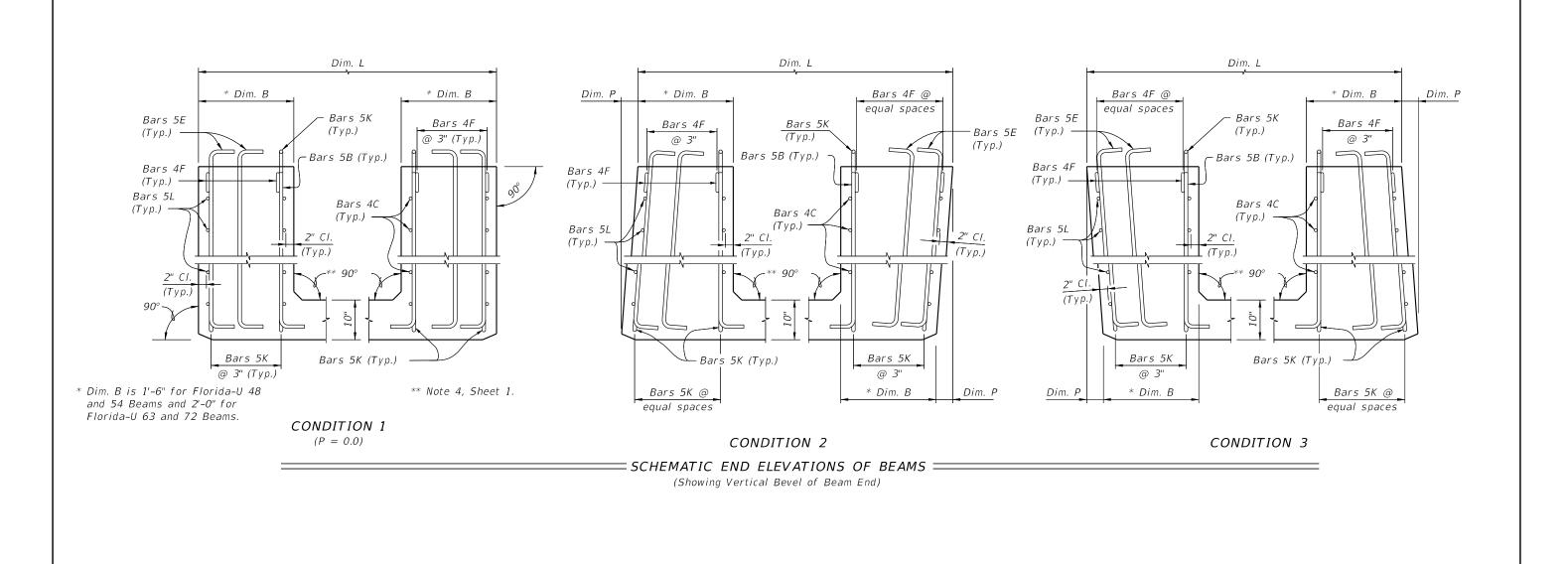
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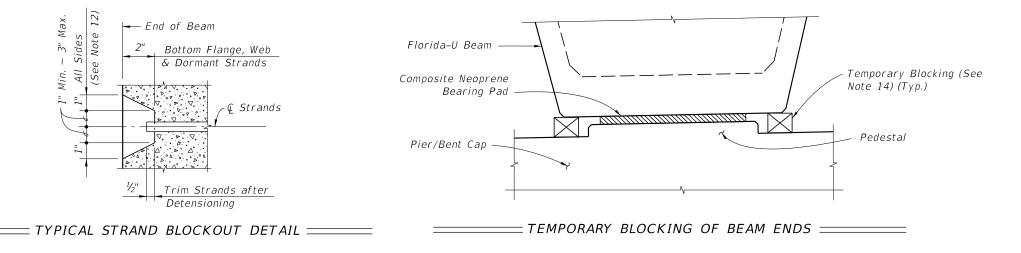


FY 2021-22 STANDARD PLANS

BEAM NOTES

- 1. Work this Index with the Florida-U Beam Standard Details (Index 450-248, 450-254, 450-263 and 450-272) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum. Maximum aggregate size is a No. 67.
- 4. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 5. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 6. Tie Bars 5K to the fully bonded strands in the bottom row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. For beams without skewed ends or vertically beveled end conditions (see Note 8) the Engineer may approve the use of deformed WWR in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M. The spacing and sizes of deformed WWR must match the reinforcing sizes shown on the Florida-U Beam Standard Details sheets.
- 8. For Beams with vertically beveled end conditions, where "Dim. P" exceeds 1", place Bars 5E, and the first Bars 4F and 5K parallel to the end of the beam. Fan the remaining Bars 4F and 5K within the limits of "Dim. B" (End Diaphragm) at equal spaces until vertical.
- 9. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any anchorage devices or other required embedded hardware.
- 10. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing the beam from casting bed.
- 11. Place drains pipes adjacent to each web at each beam end (four drains per beam).
 - Drain Pipe: 2" NPS Schedule 80 PVC.
 - Cover, wrap and secure wire screen around the end of the pipe prior to casting. Extend screen a minimum of 1" down the pipe sides.
 - Provide removable pipe plugs during casting. Remove plugs from the inside of
- 12. Protection of Strands:
 - A. Provide a 2" deep recess around all strands (including dormant) or strand groups. Extend the recessed blockout to the web face and bottom of the flange for the bottom row of strands.
 - After detensioning, cut strands $\frac{1}{2}$ " from recessed surface and fill the blockout to protect strands with Type F-2 or Q Epoxy Compound in accordance with Specification Section 926.
- 13. Use Stay-In-Place metal deck forms inside the beams.
- 14. Prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations during deck placement. Leave temporary blocking and bracing in place for a minimum of four days after the deck is placed.
- 15. Based on the deck forming system and deck placement sequence, evaluate and provide any required temporary bracing between the U Beams.





REVISION 11/01/16

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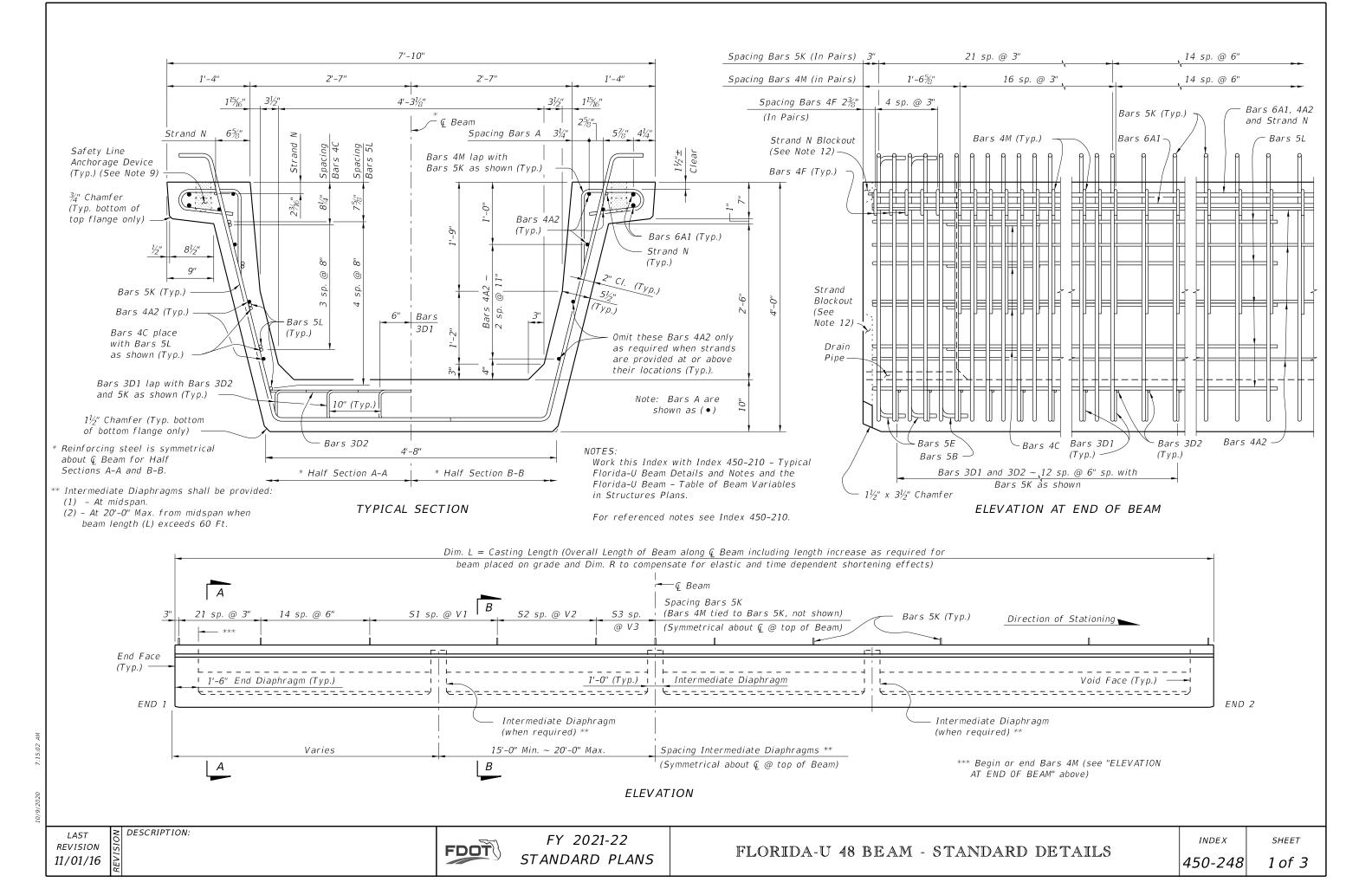
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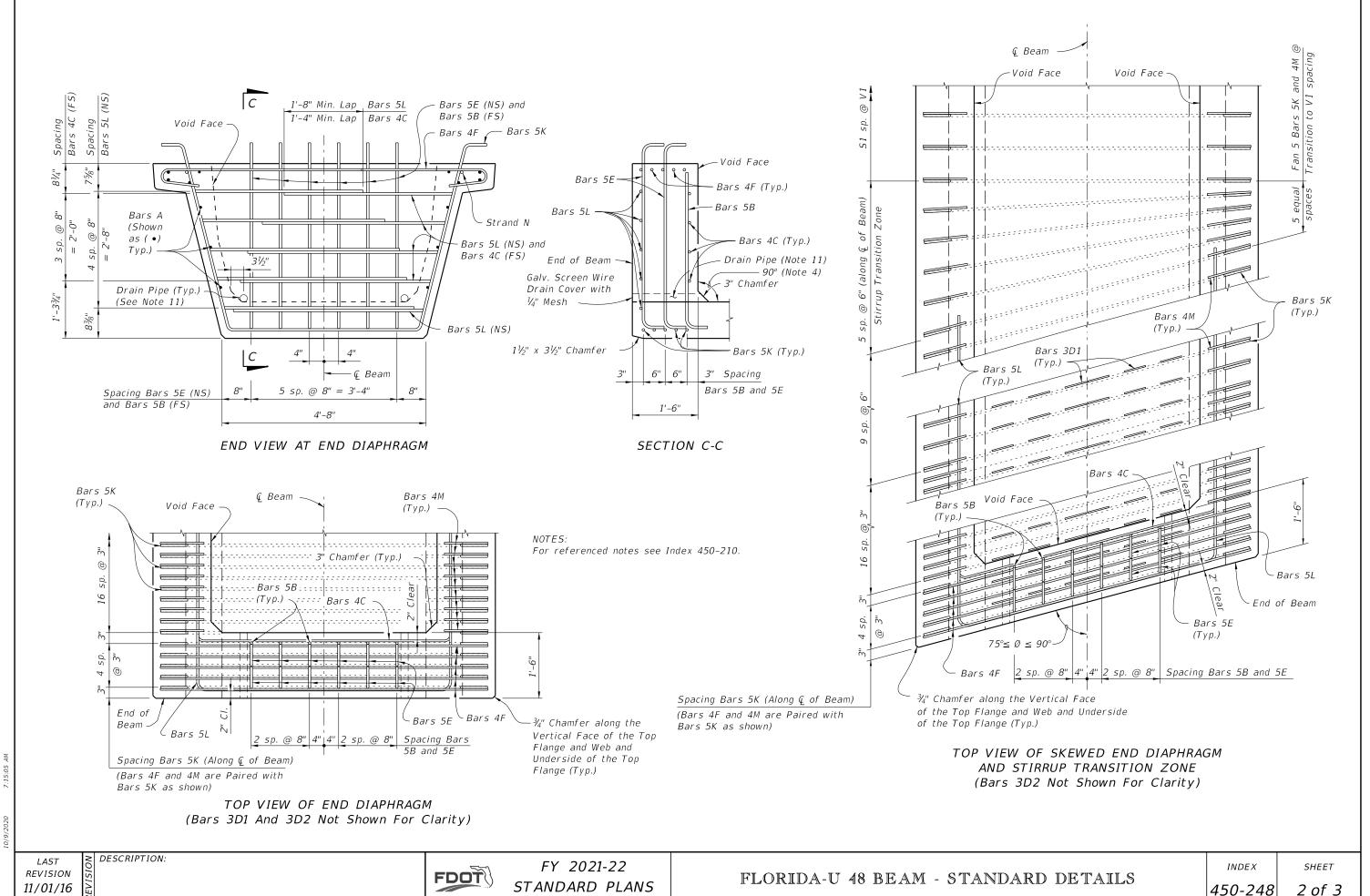
FLORIDA-U BEAM - TYPICAL DETAILS & NOTES

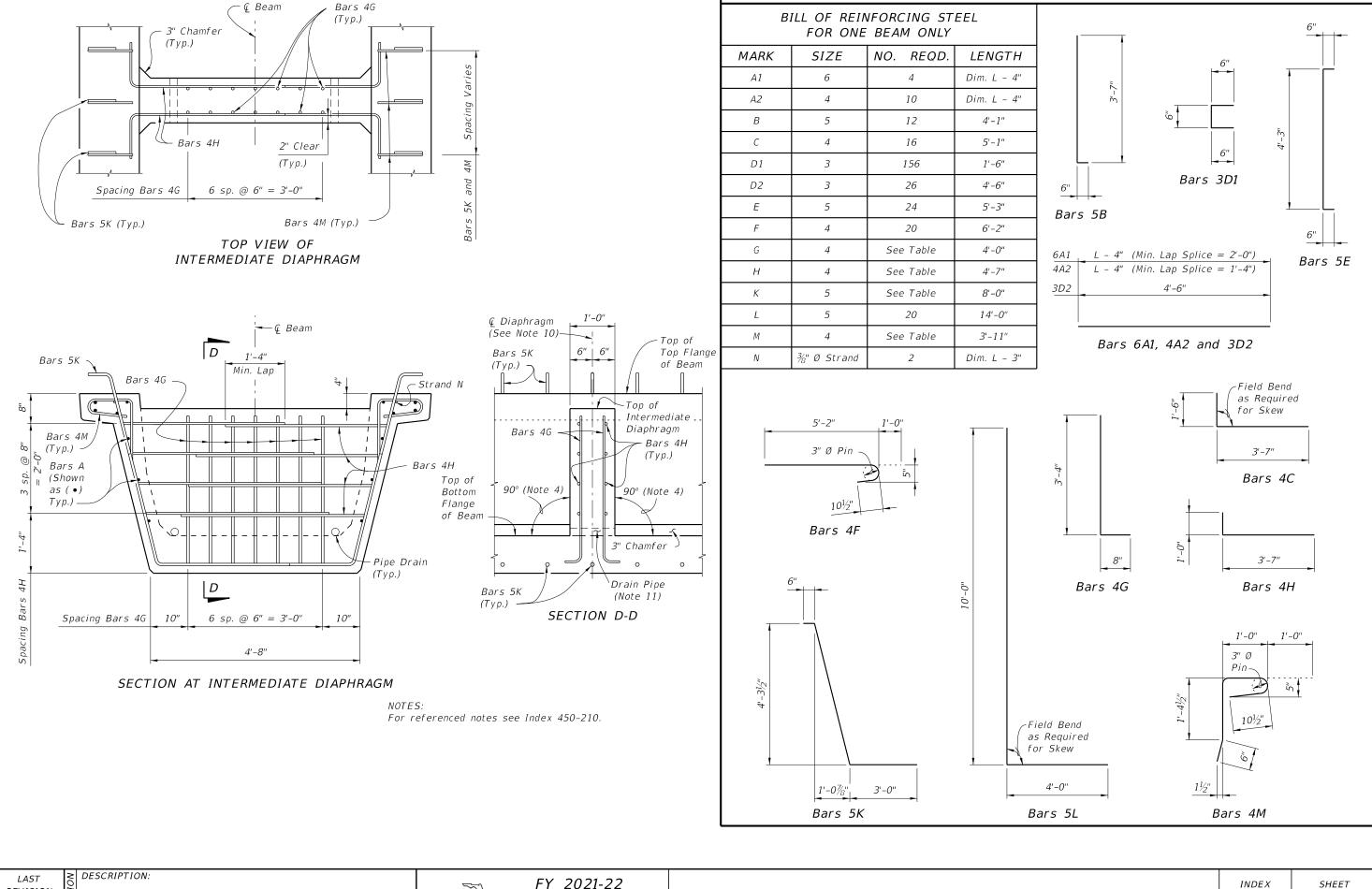
INDEX 450-210

SHEET

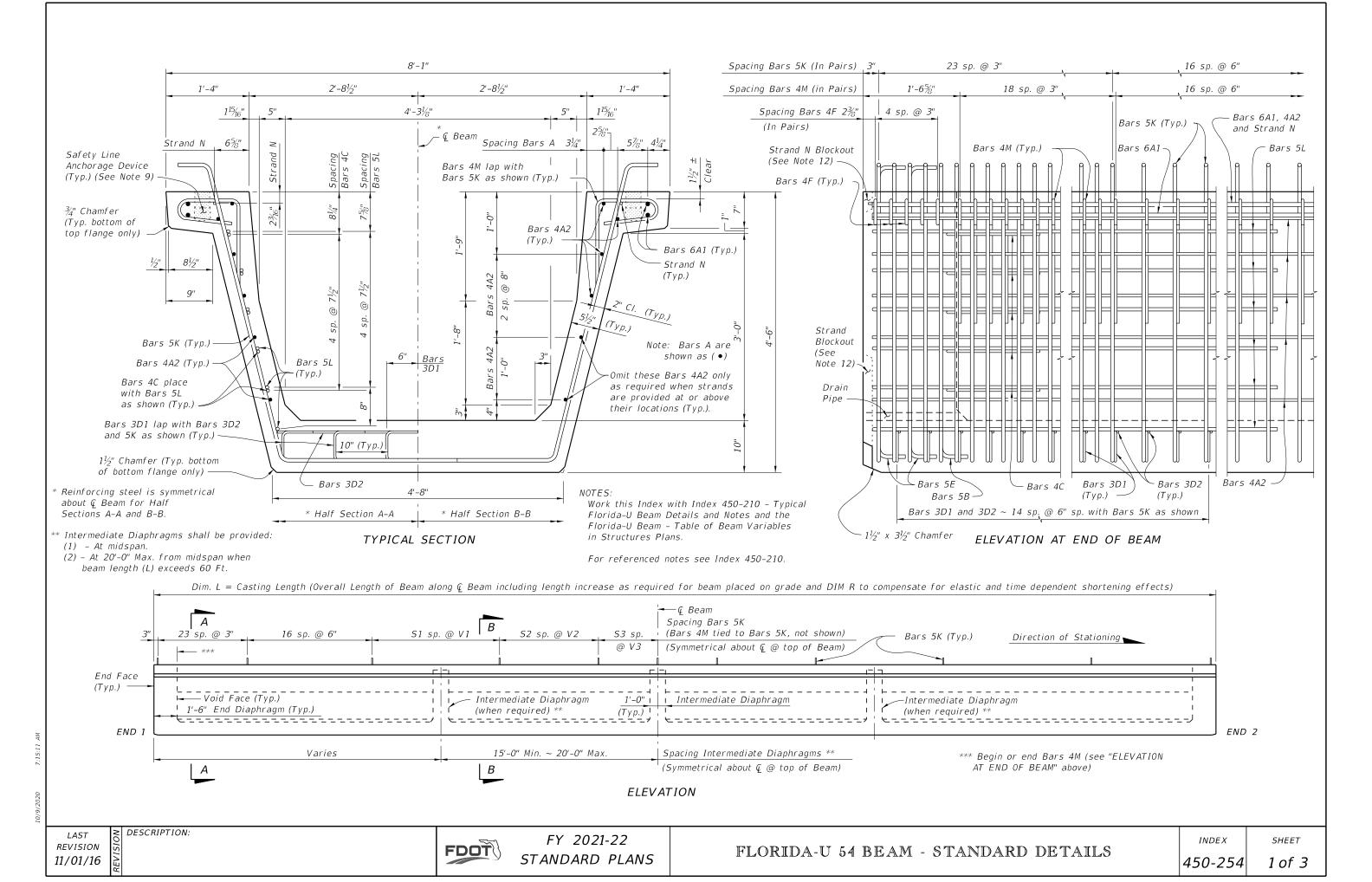
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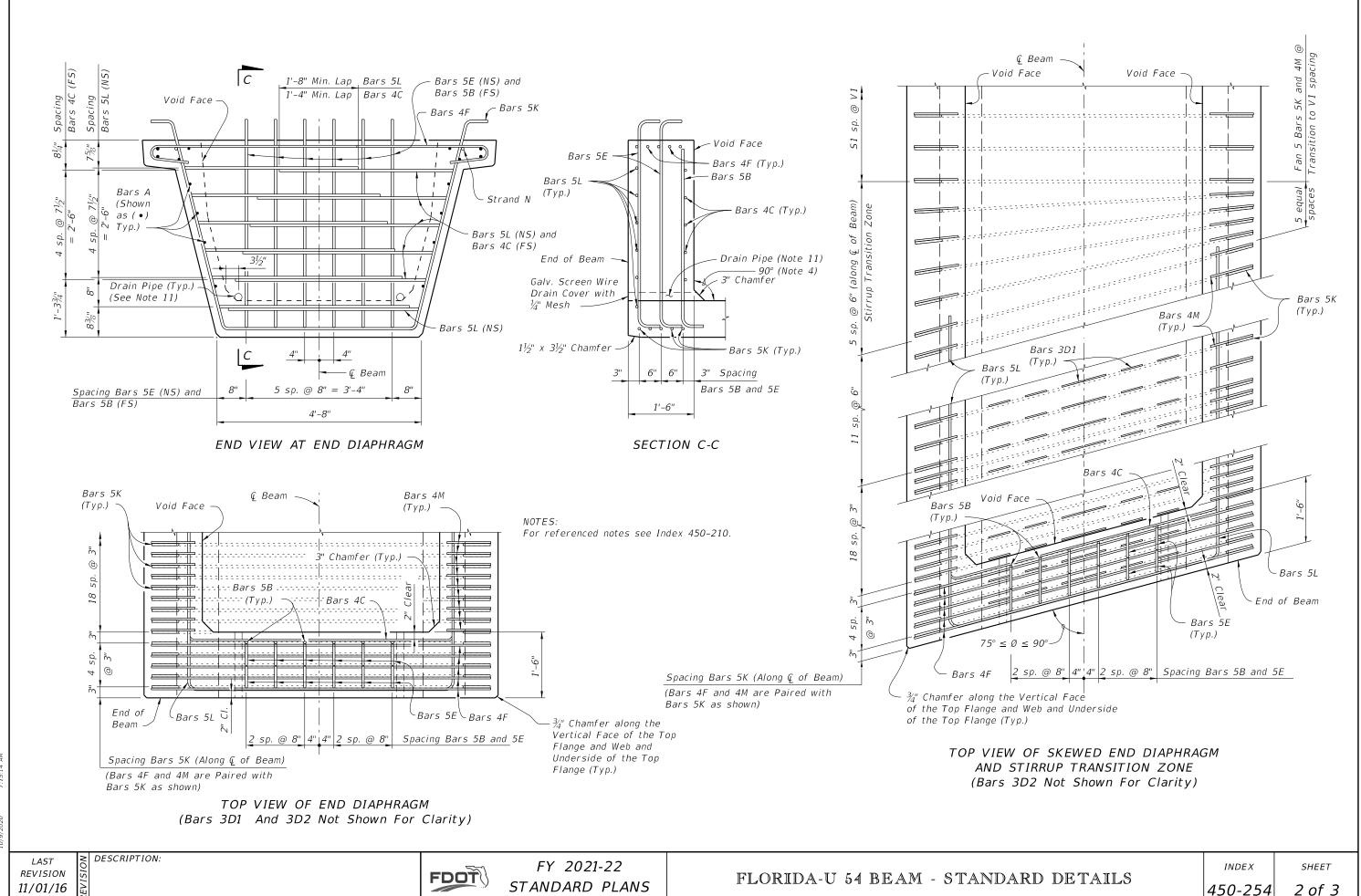


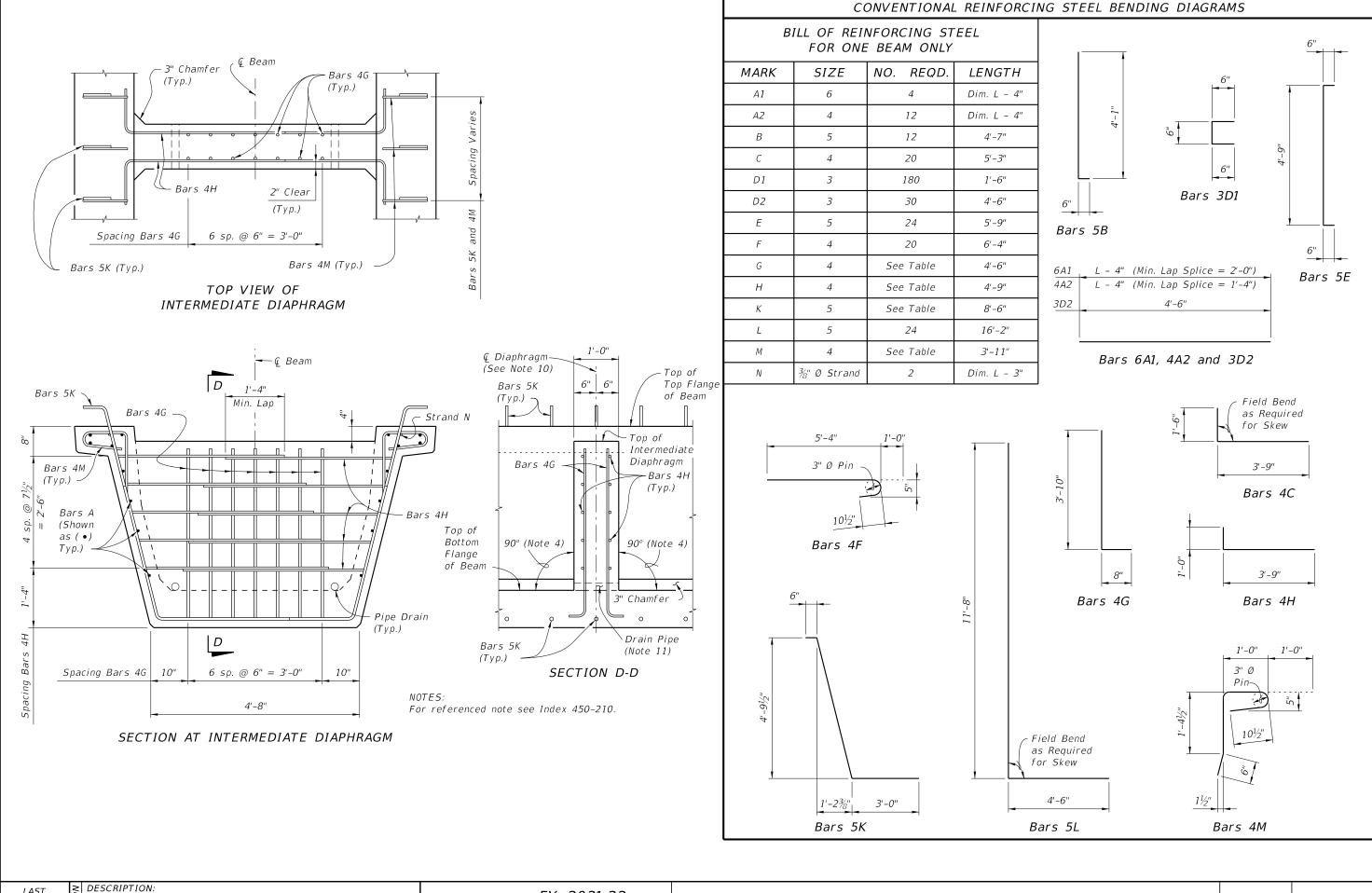




REVISION 11/01/16 CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS



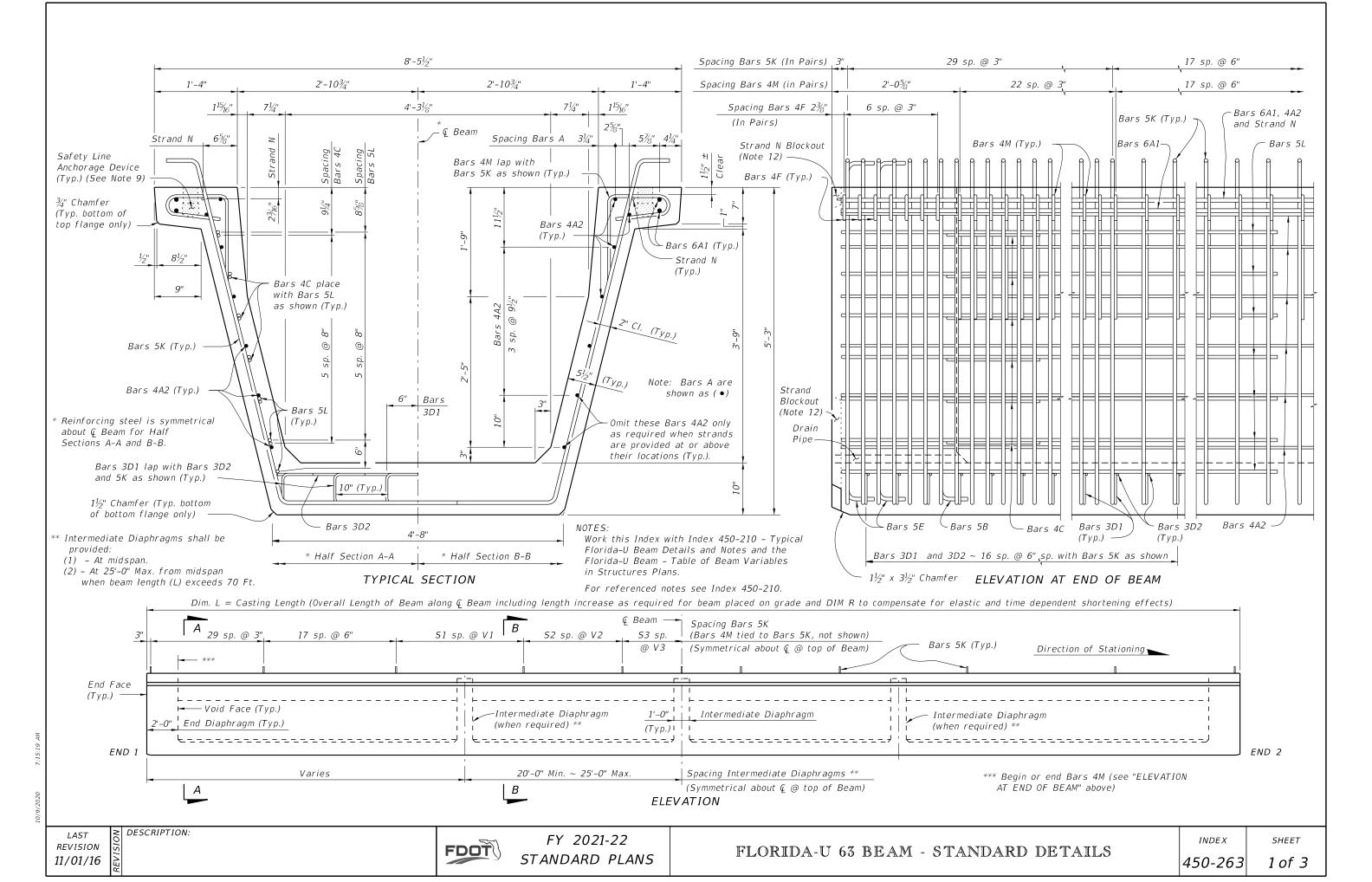


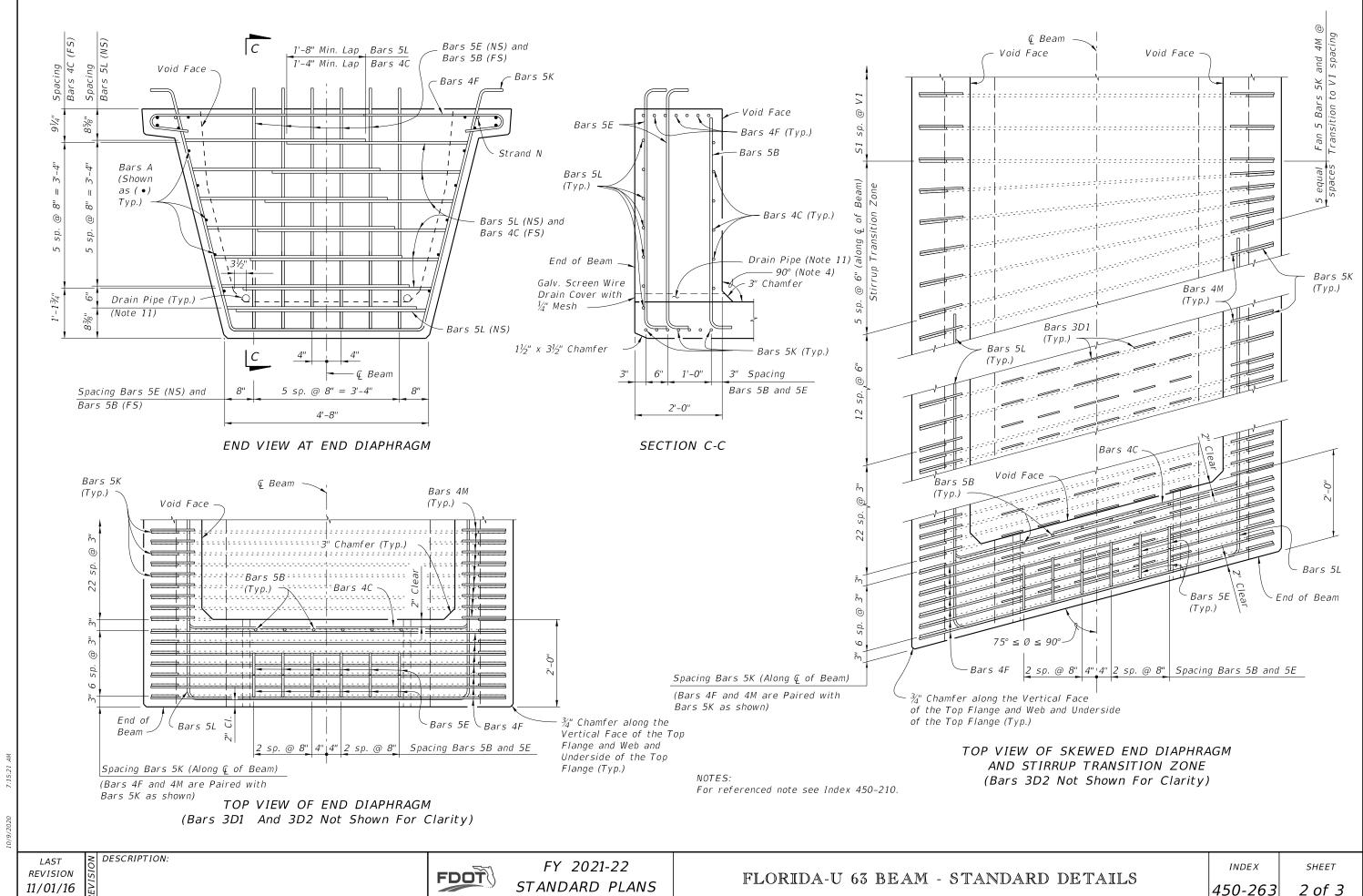


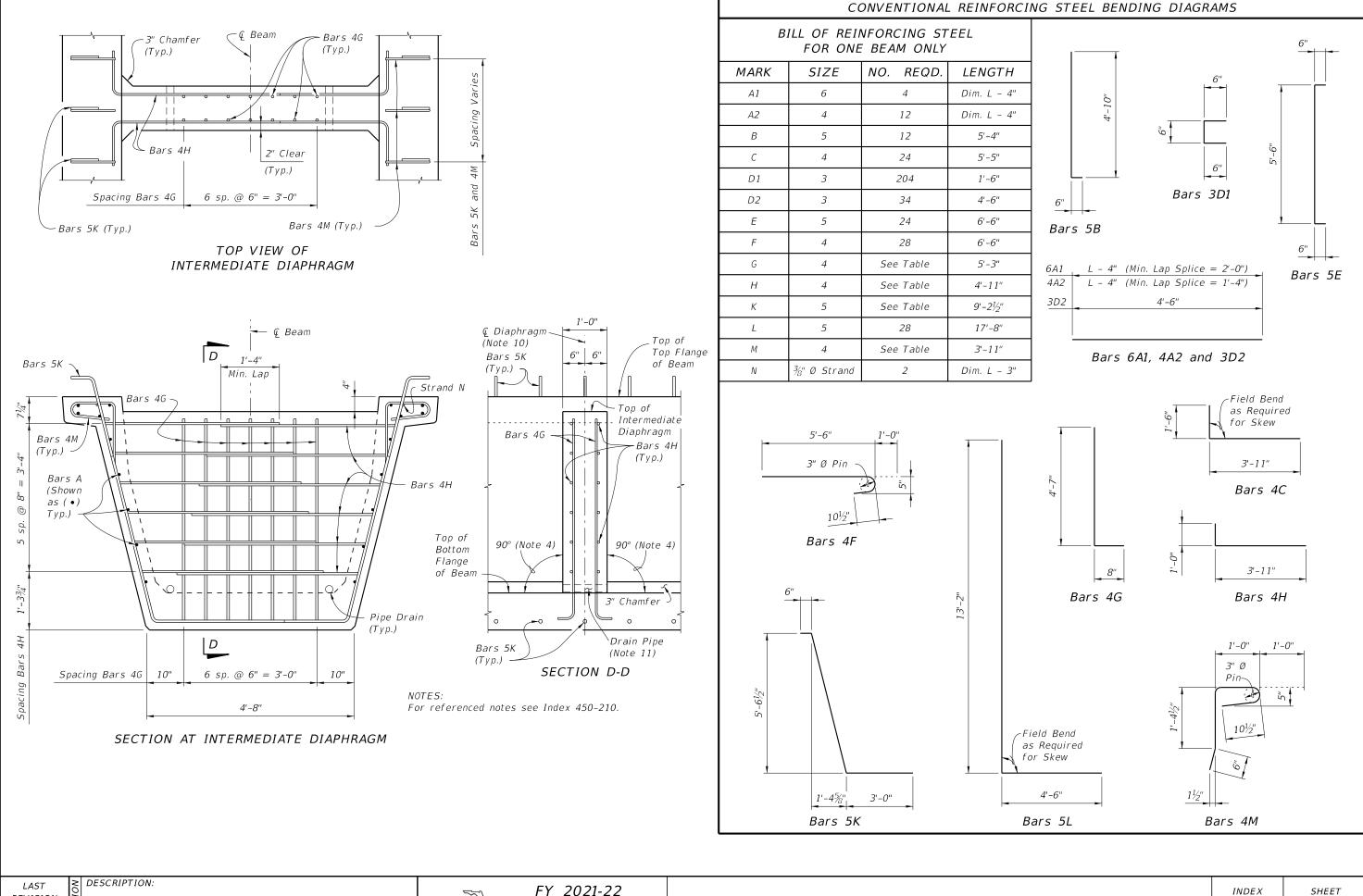
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LAST REVISION 11/01/16

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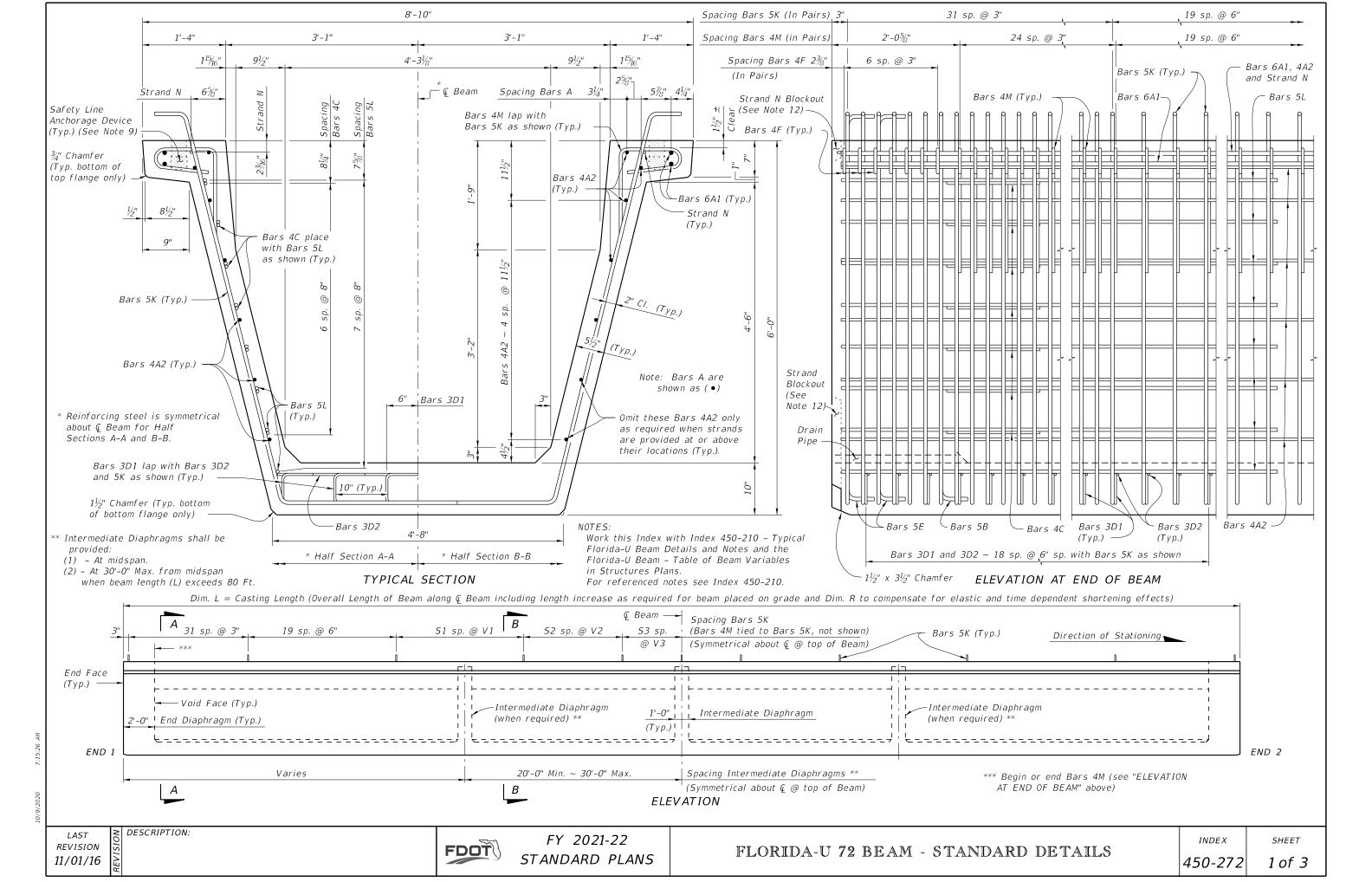


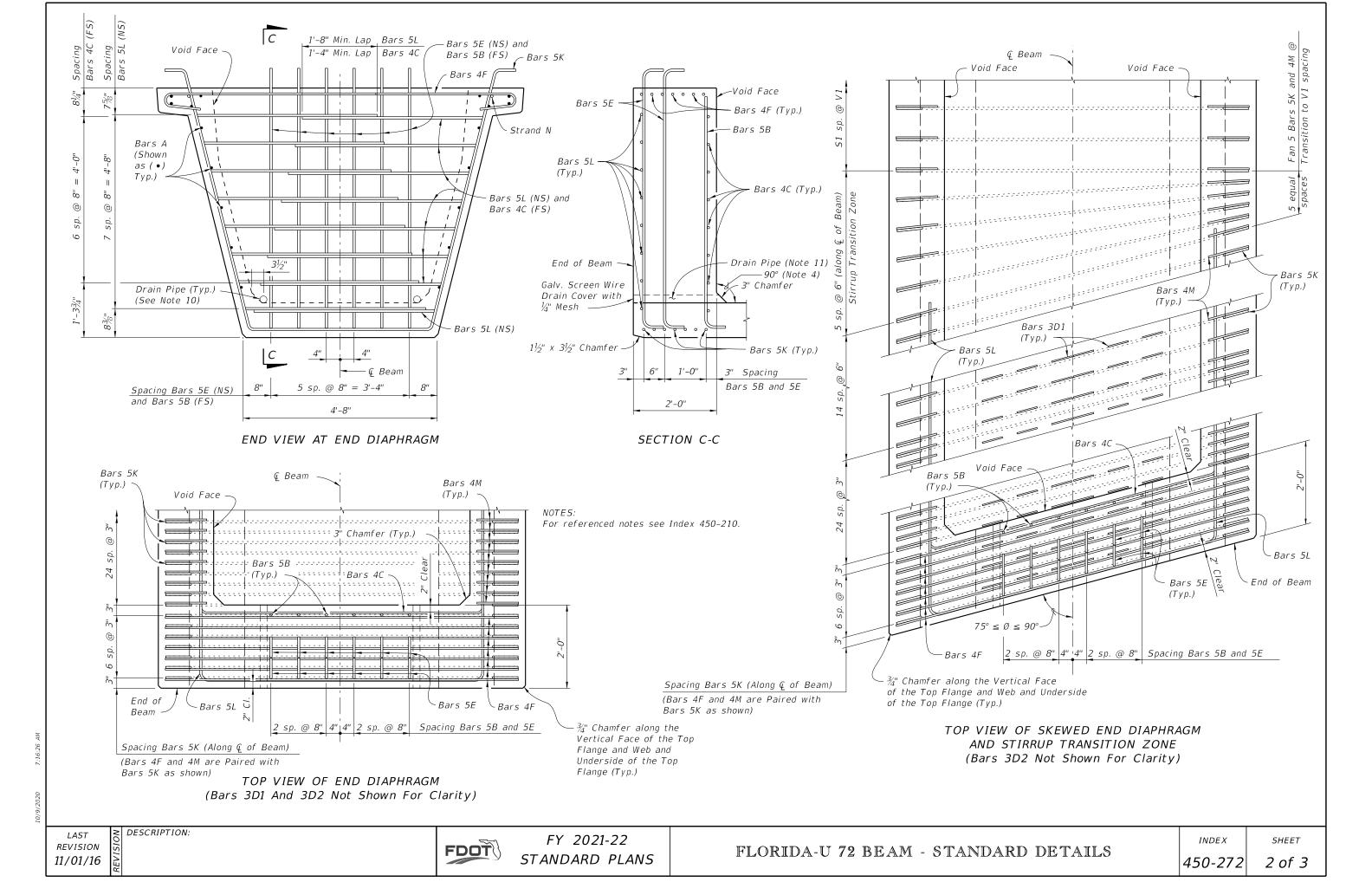
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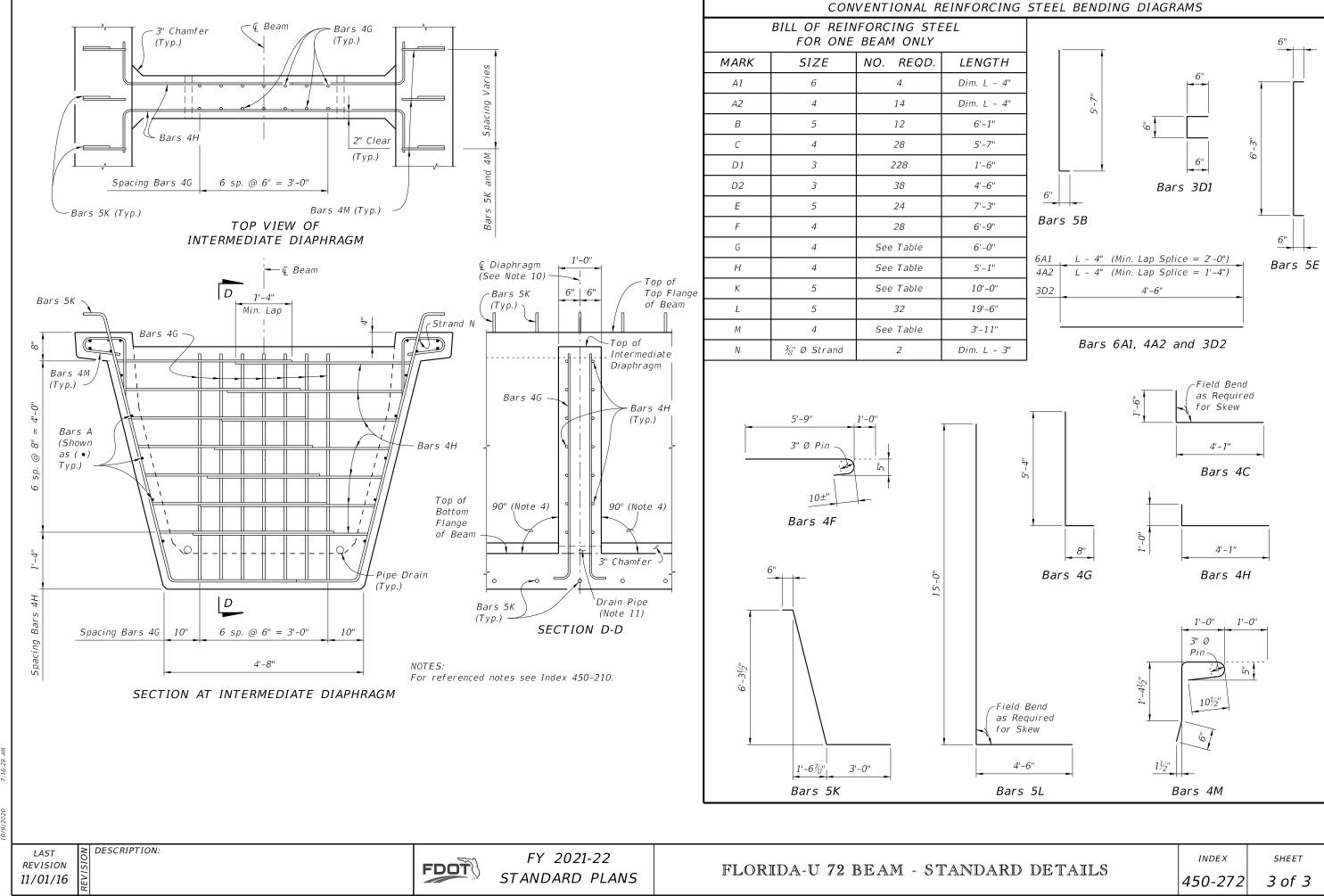
REVISION 11/01/16

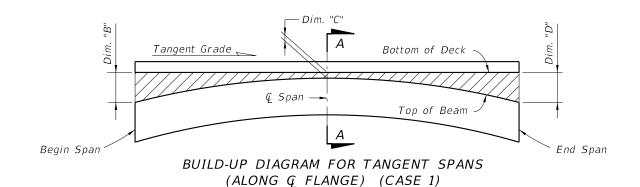
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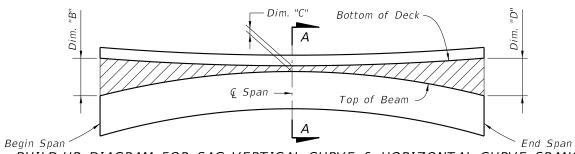
FY 2021-22 STANDARD PLANS



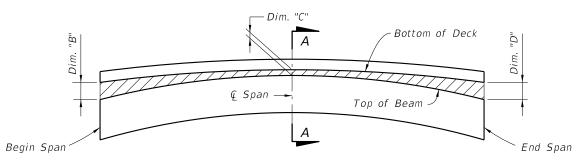




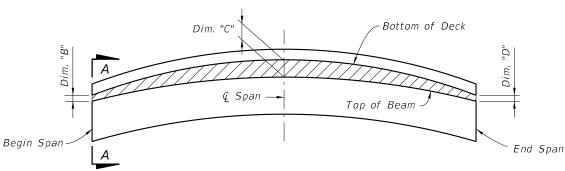




BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q FLANGE) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG G FLANGE) (CASE 3)

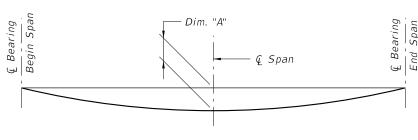


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG G FLANGE) (CASE 4)

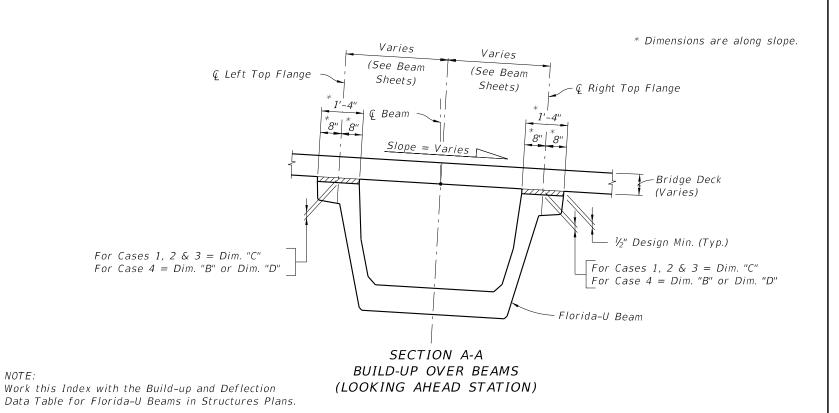
BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM (ALONG G BEAM)



DESCRIPTION: **REVISION** 07/01/15

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FY 2021-22 STANDARD PLANS

FLORIDA-U BEAMS - BUILD-UP & DEFLECTION DATA INDEX

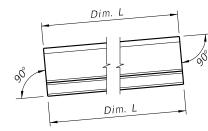
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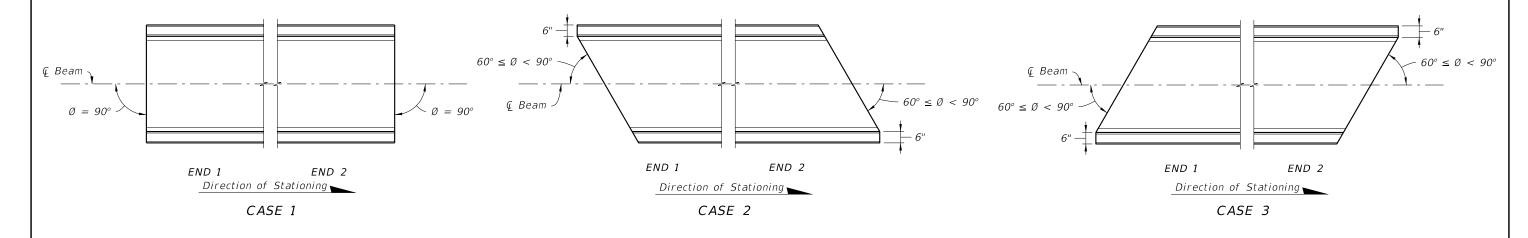
450-299

FABRICATION NOTES

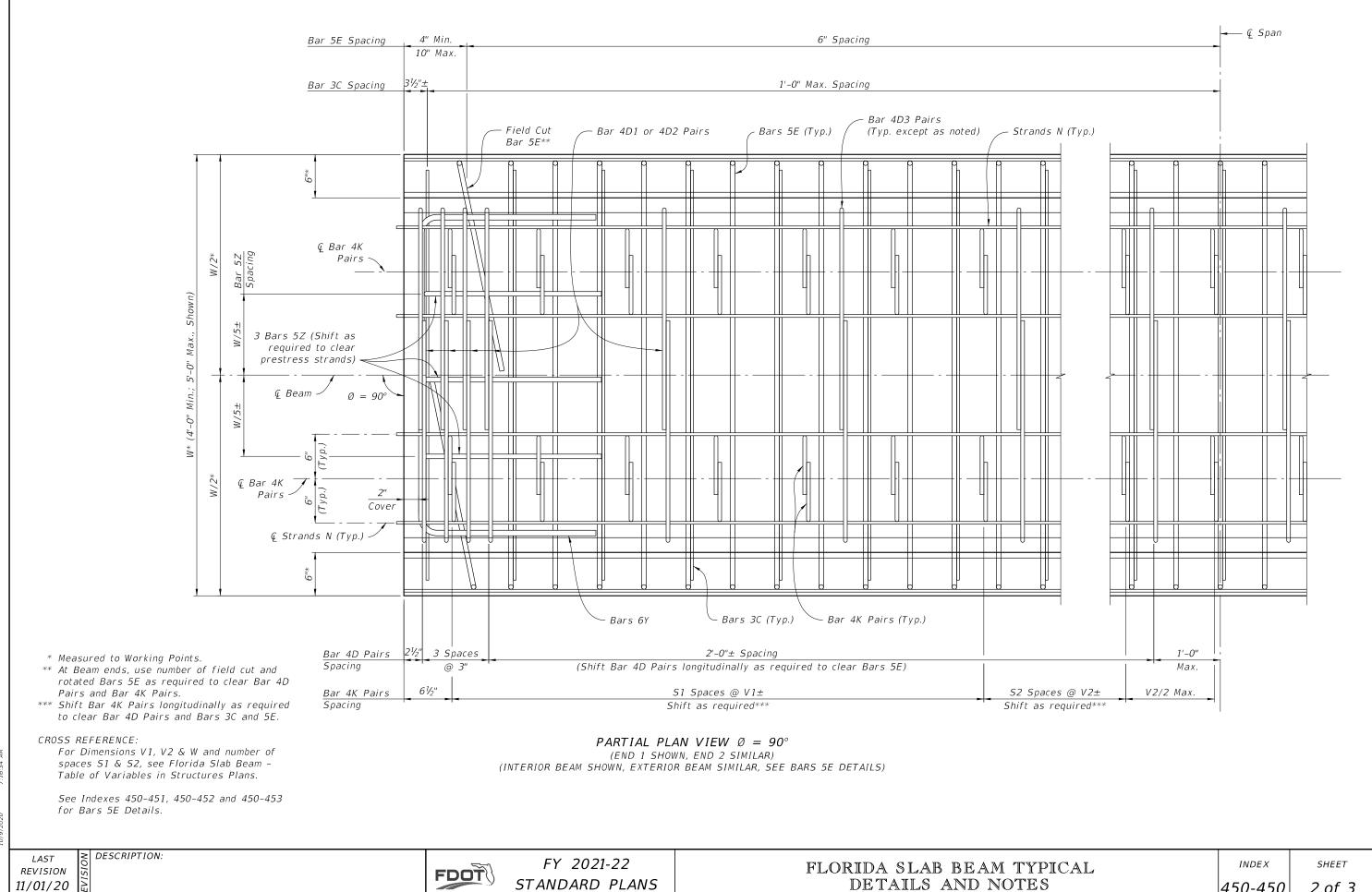
- 1. The abbreviated FSB designation for depth and width is FSB "depth" x "width", e.g. FSB 12 x 48.
- 2. All bar dimensions are out-to-out.
- 3. Strands N shall be ASTM A416, Grade 250 or 270, 3/8" Ø or larger strands, stressed to 10,000 lbs. each.
- 4. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 5. For referenced Dimensions, Angles and Case Numbers, see Florida Slab Beam - Table of Variables in Structures Plans.
- 6. Bars 4D1 & 6Y1 correspond to END 1, and 4D2 & 6Y2 correspond to END 2.
- 7. Bars 5E1 correspond to interior FSBs, and 5E2 correspond to exterior FSBs.
- 8. Rake the top surface of the Slab Beams transversely to provide a roughened surface with $\frac{1}{4}$ " amplitude.
- 9. Embedment of Safety Line Anchorage Devices are permitted to accommodate full protection systems. See shop drawings for details and spacings.

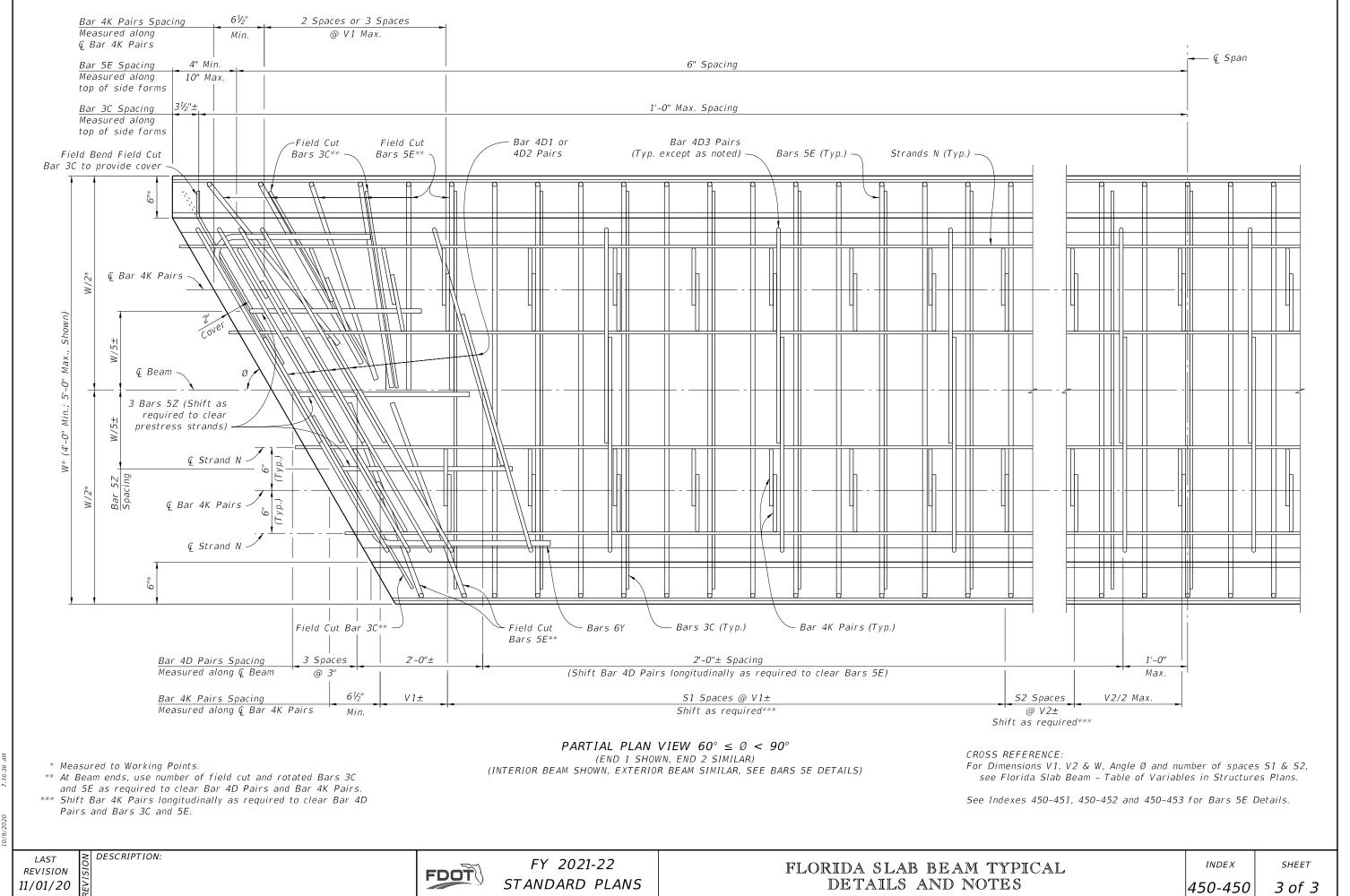


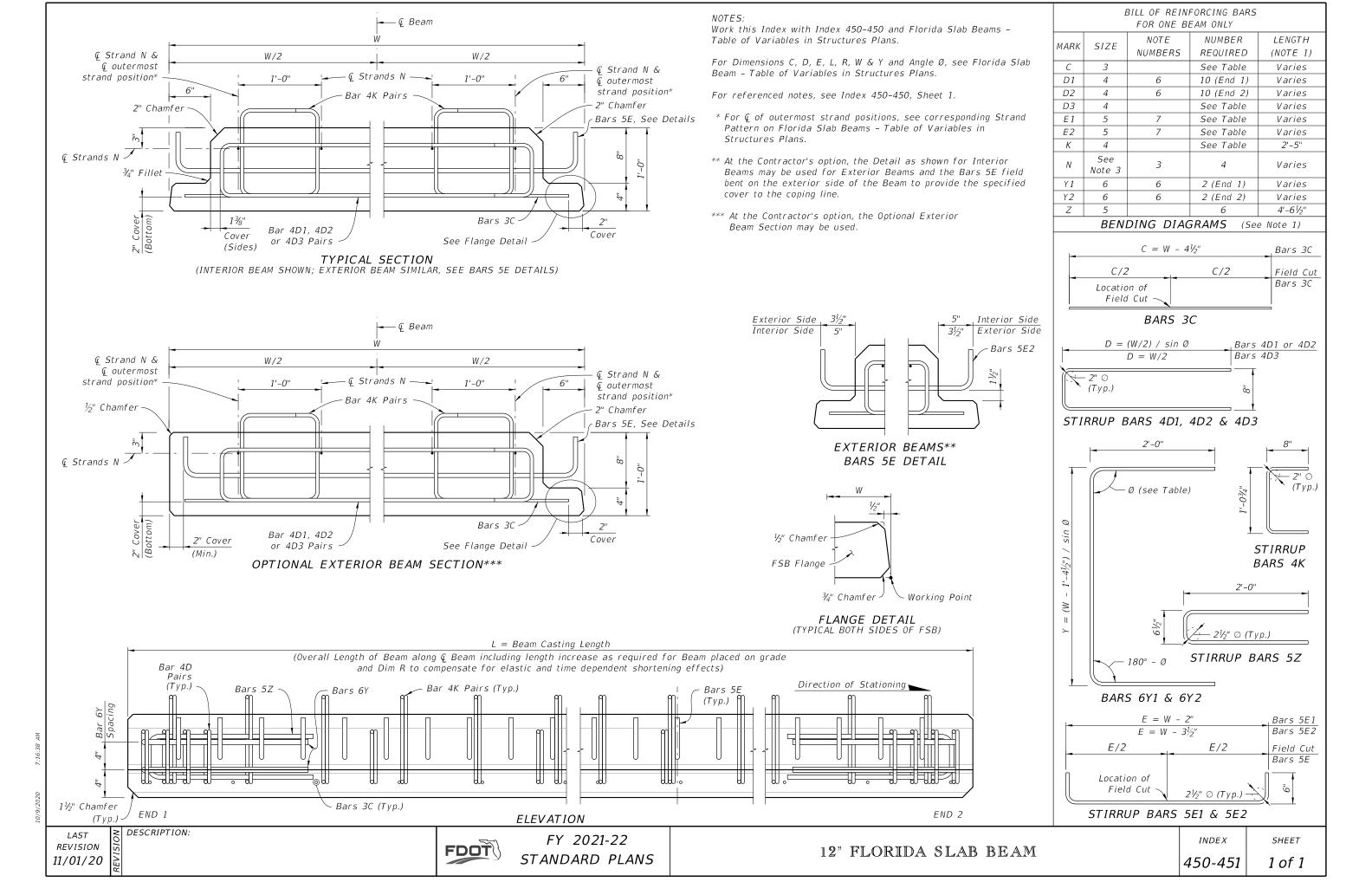
SCHEMATIC SIDE ELEVATION OF BEAM (Beam on a Positive Grade shown; Beam on a Negative Grade or Horizontal Grade similar.)

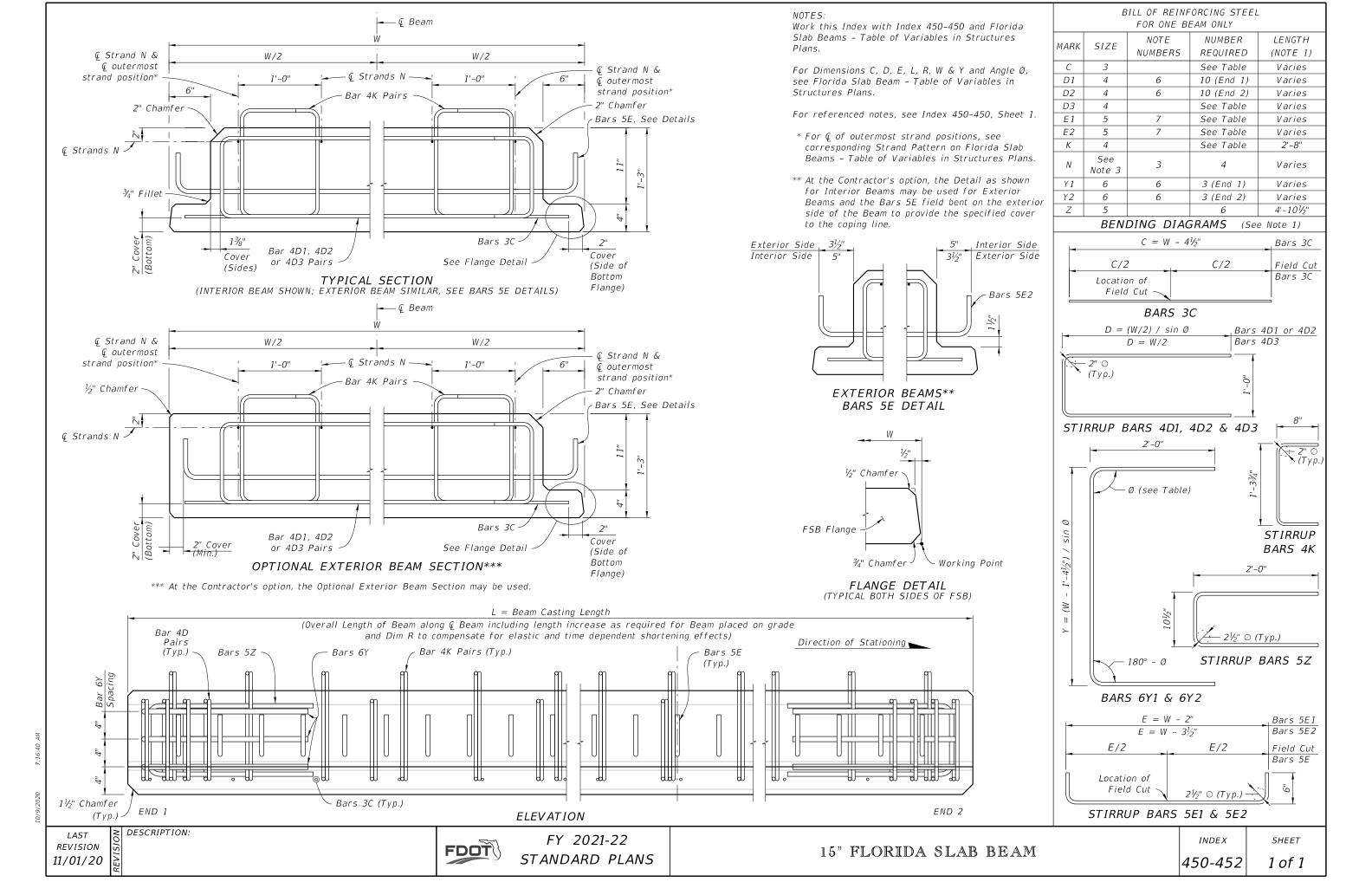


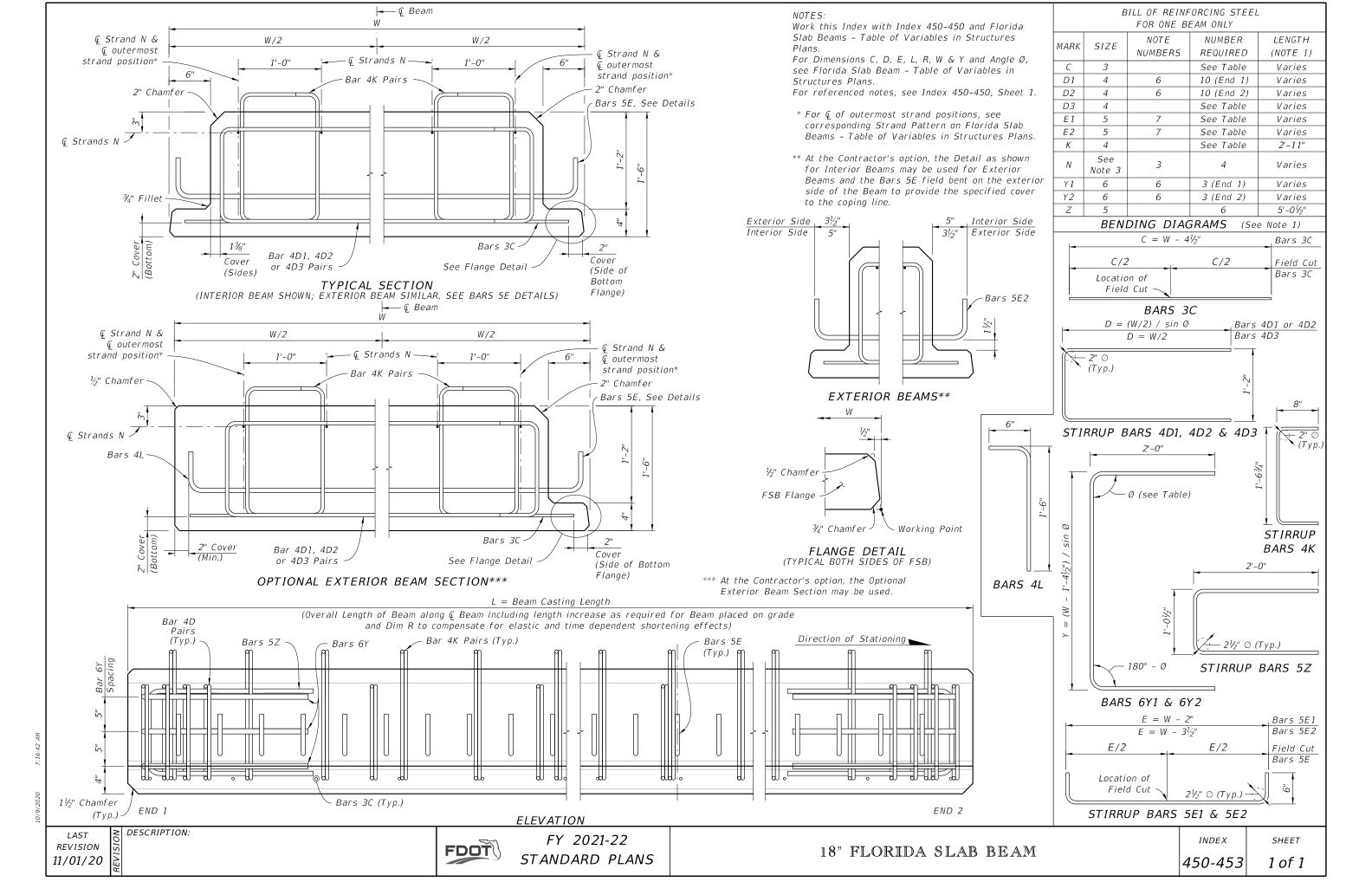
SCHEMATIC PLAN VIEWS AT BEAM ENDS

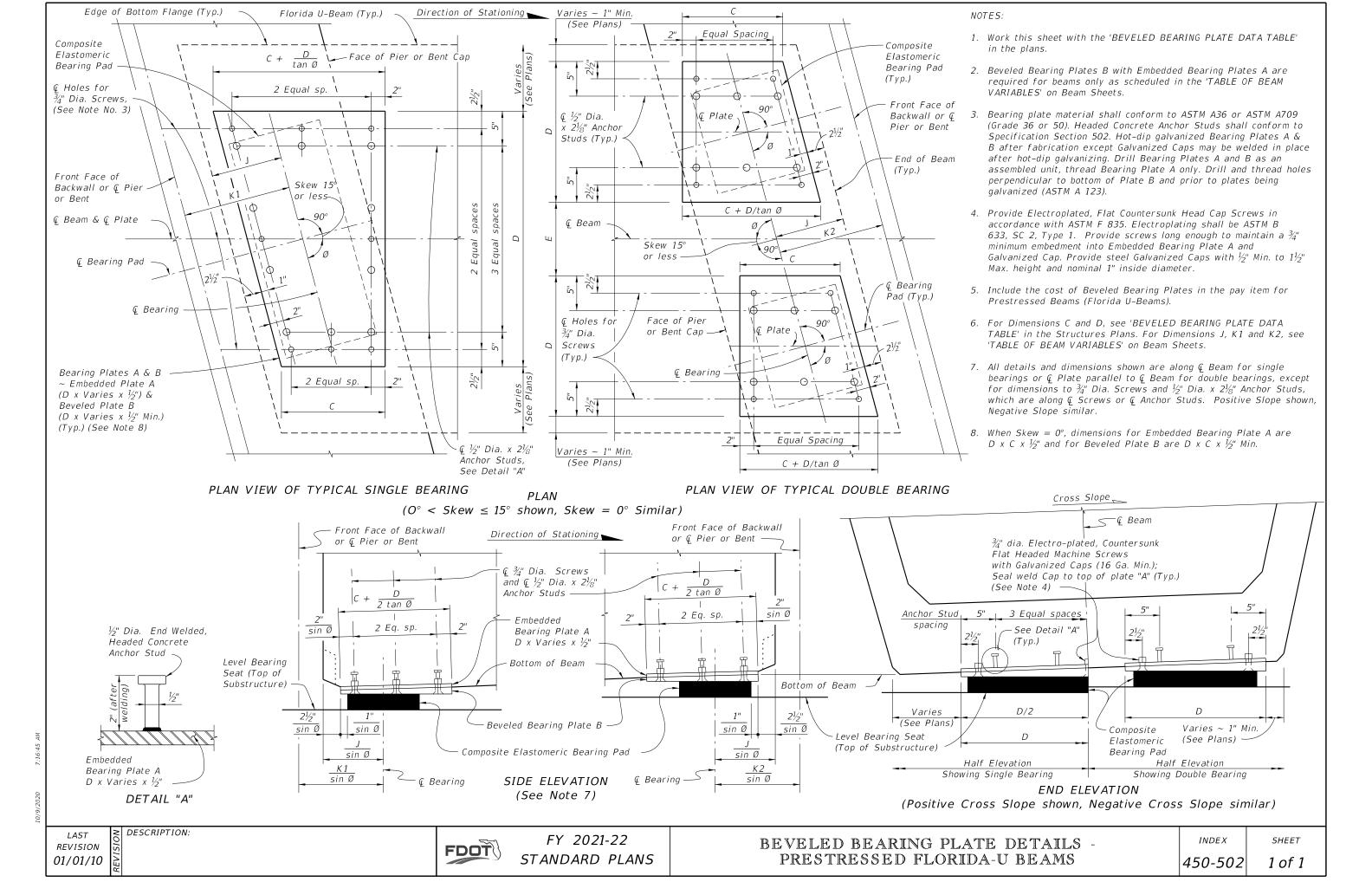


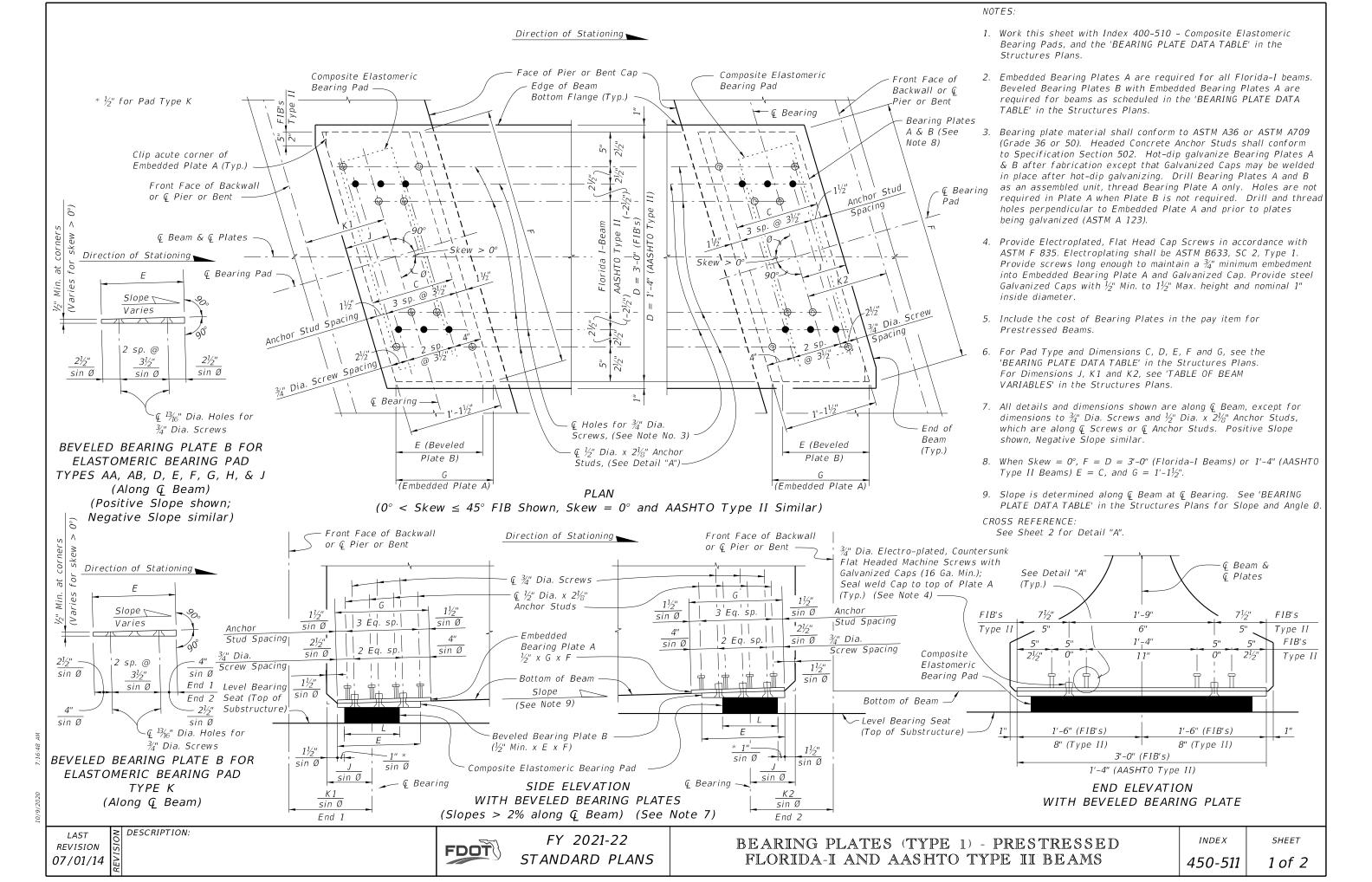


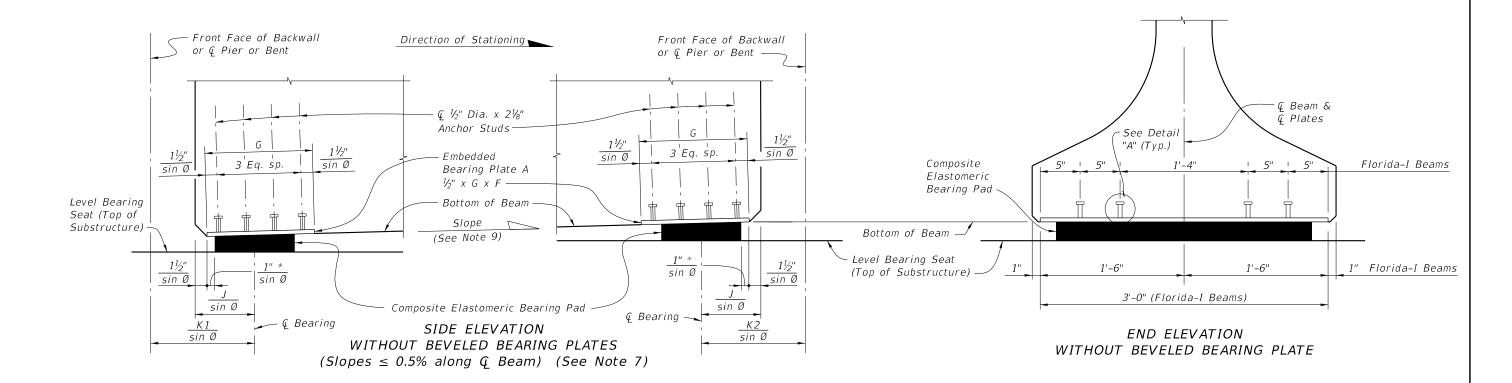




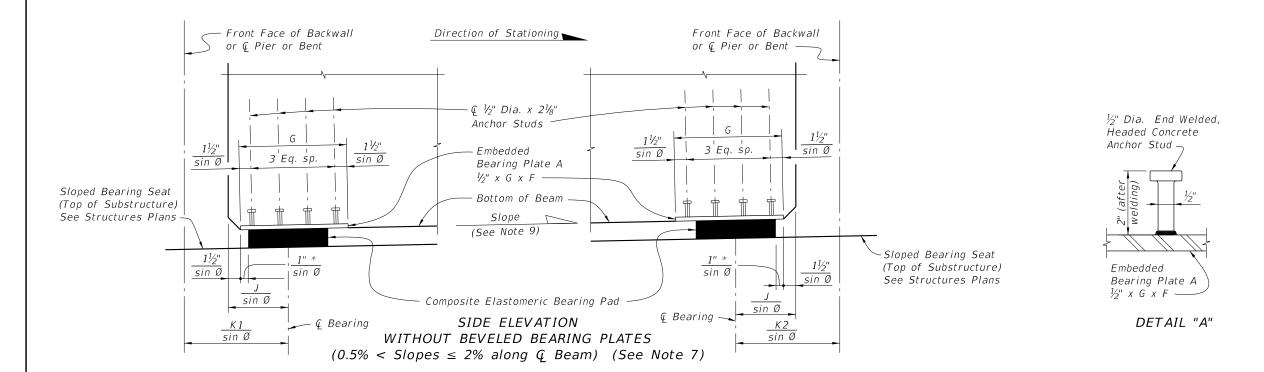








* ½" Pad Type K

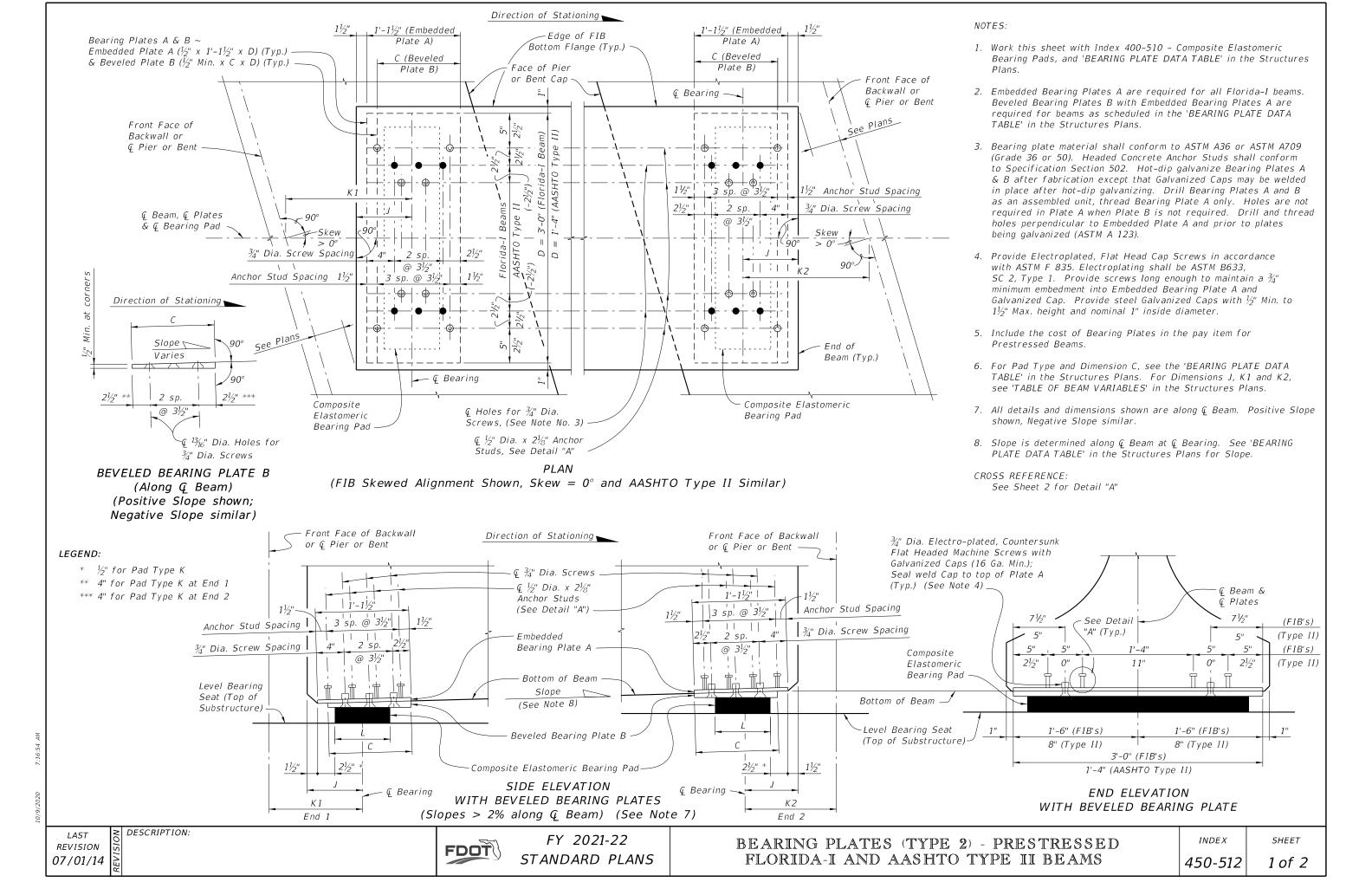


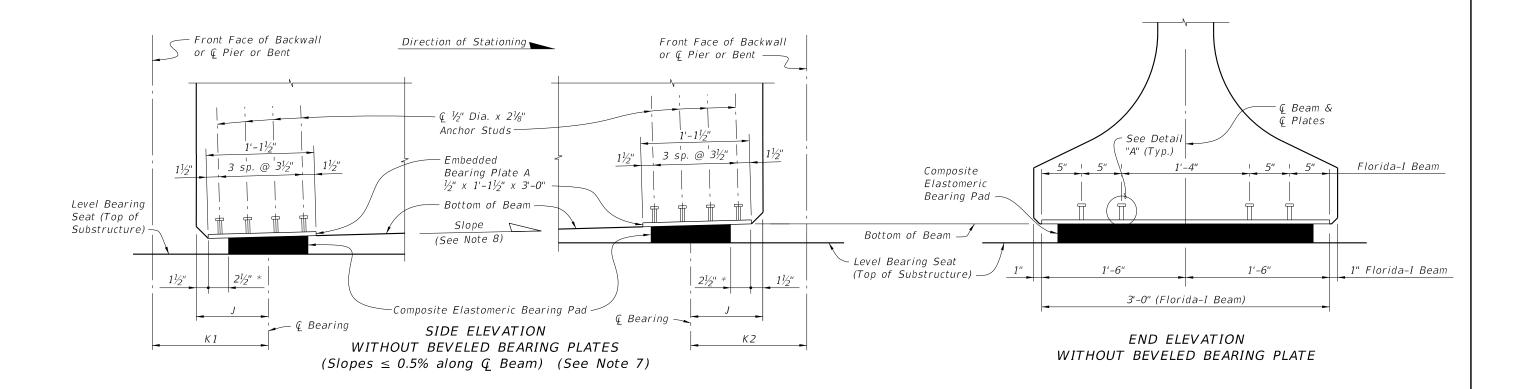
CROSS REFERENCE:
See Sheet 1 for Notes.

DESCRIPTION:

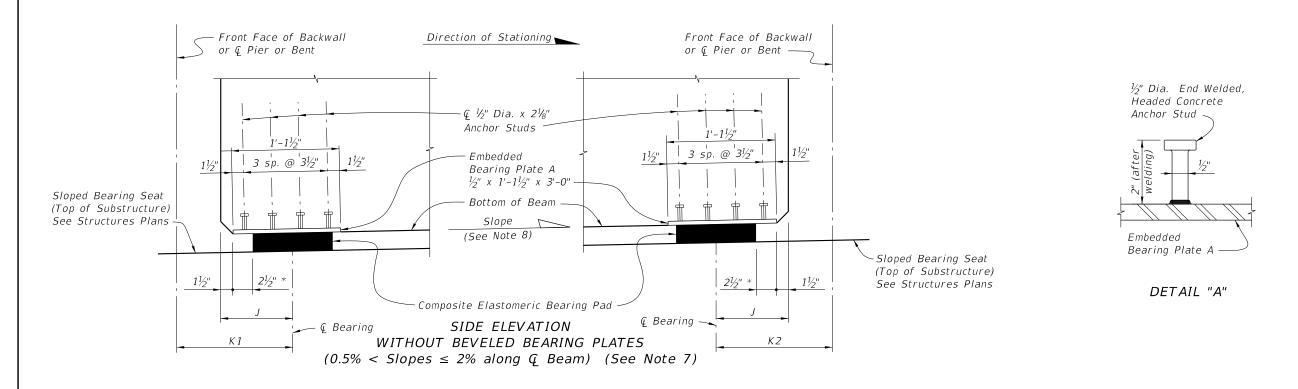
LAST REVISION 11/01/20

FDOT





* ½" for Pad Type K



CROSS REFERENCE:
See Sheet 1 for Notes.

LAST REVISION 07/01/14

DESCRIPTION:

FDOT

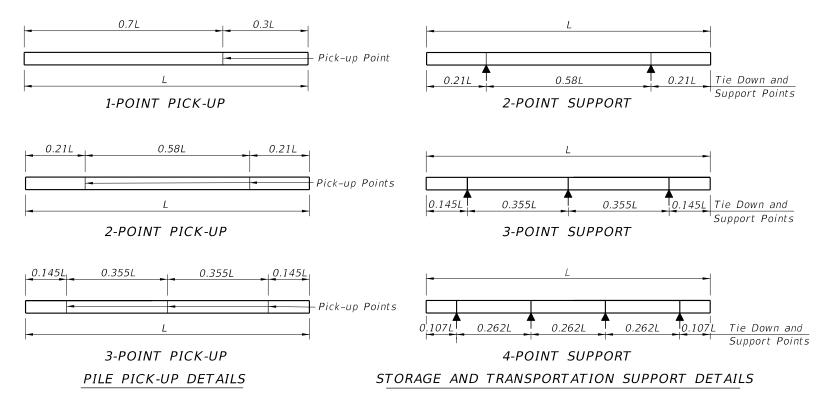
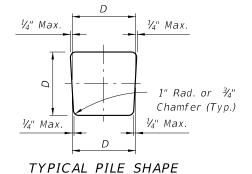
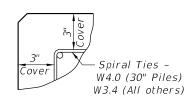


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = 0	Square	e Pile	Size	(inches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point				
	69	75	85	98	124	2, 3, or 4 point	2 Point				
	99	107	121	140	178	3 or 4 point	3 Point				





DETAIL SHOWING TYPICAL COVER

PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V (Special), except use Class VI for High Moment Capacity Pile (Index 455-031).
 - B. High Capacity Splice Collar: Class V (Special).
 - C. See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
 - B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
 - A. Bars: Meet the requirements of Specification Section 415.
 - B. Prestressing Strands: Meet the requirements of Specification Section 933.
 - Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
- B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 962. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

7:17:01 AM

FOR MOLD FORMS



- 1. For Sections D-D, E-E, & F-F see Index 455-012 thru 455-030 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- 3. In cases where pile splices are desired due to length limitations in shipping and/or handling, the "Drivable Preplanned Prestressed Precast Splice Detail" shall be used.

 Mechanical Pile Splices contained on the Approved Products List (APL) may also be used.
- 4. When preformed dowel holes are utilized, the 1" spiral tie pitch shall be continued to 4'-0" below the head of the pile, See Index 455-018, 455-020 & 455-024. Preformed holes shall utilize either removable preforming material or stay-in-place corrugated galvanized steel ducts. Stay-in-place ducts shall be fabricated from galvanized sheet steel meeting the requirements of ASTM A653, Coating Designation G90, 26 gauge. Ducts shall be 2" diameter with a minimum corrugation (rib) height of 0.12 in. Ducts shall be fabricated with either welded or interlocked seams. Galvanizing of welded seams will not be required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 10 Dowels into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.

M

(Typ.)

D

1" Cover

at End

No. 10 Dowels

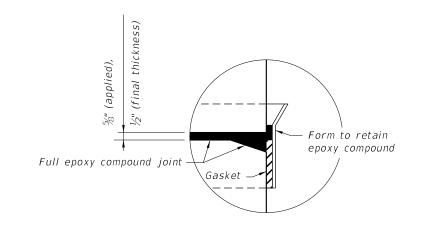
Mini

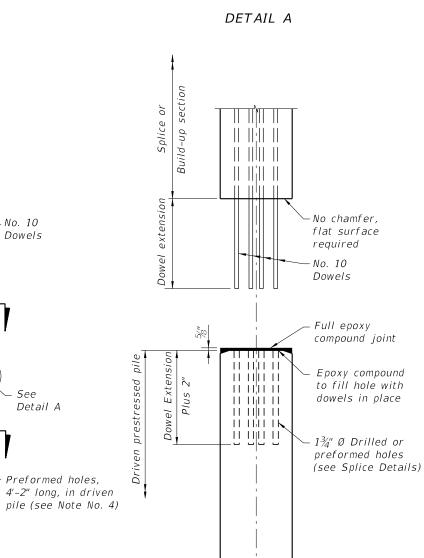
Full length of

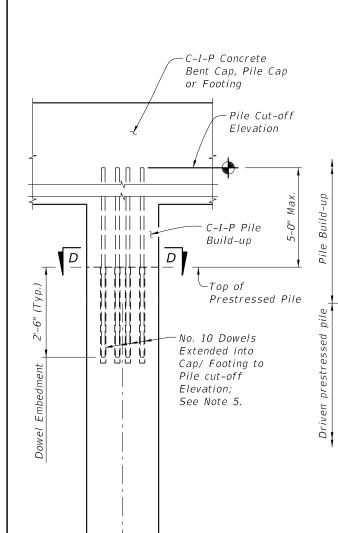
Build-up

D

Detail A







NON-DRIVABLE UNFORESEEN REINFORCED PRECAST PILE BUILD-UP DETAIL

. U U'U U

DRIVABLE UNFORESEEN
PRESTRESSED PRECAST
PILE SPLICE DETAIL

. 6 6 6 6

E

_ No. 10

Ε

Dowels

Detail A

-0

Splice

pile

sed

Driven prestres

extension

Dowe!

DRIVABLE PREPLANNED PRESTRESSED PRECAST PILE SPLICE DETAIL

Ε

Auxiliary reinforcing

steel cast with pile. See Section F-F.

ΙE

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LAST REVISION 07/01/14

UNFORESEEN

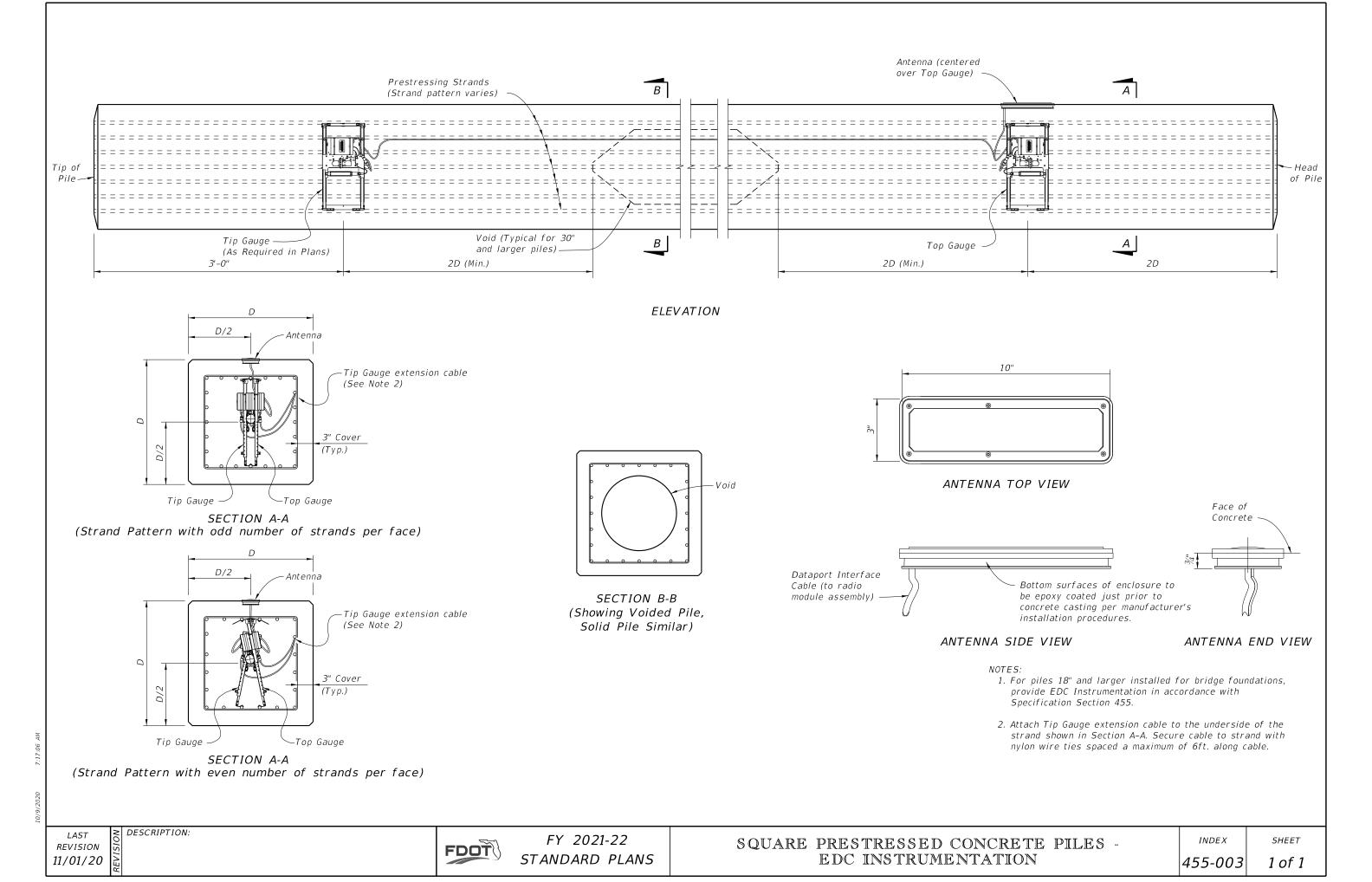
REINFORCED C-I-P

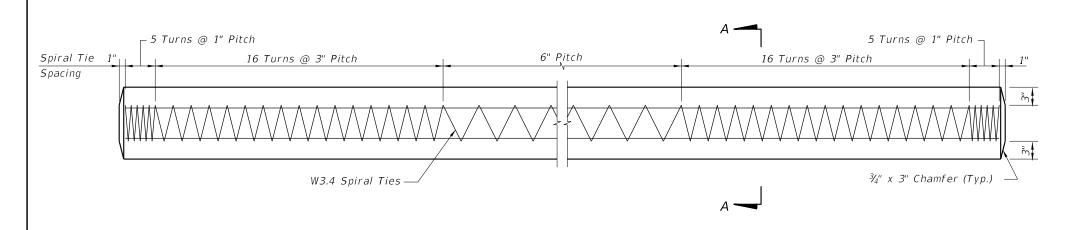
PILE BUILD-UP DETAIL



TYPICAL SPLICE BEFORE BONDING

AST O DESCRIPTION:





ALTERNATE STRAND PATTERNS

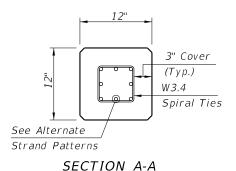
4 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

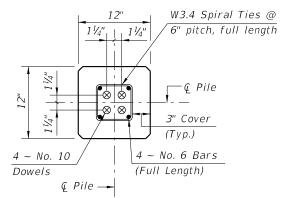
 $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 25 kips

 $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 24 kips

 $8 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 23 kips

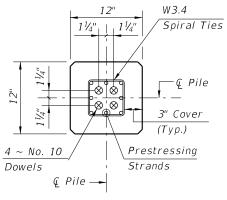
12 ~ 3%" Ø, Grade 270 LRS, at 16 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

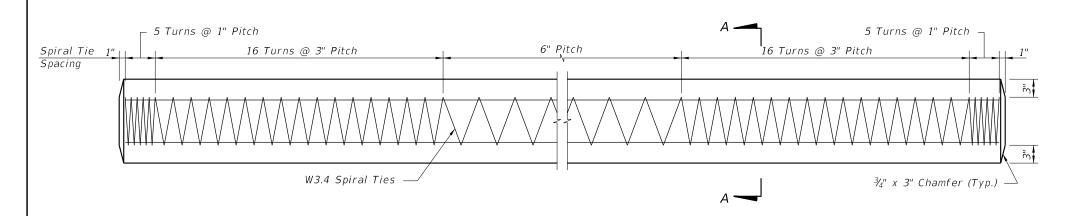
PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 450-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 -Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

DESCRIPTION:

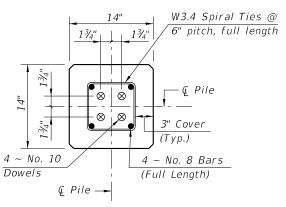


3" Cover (Typ.) W3.4 Spiral Ties See Alternate Strand Patterns

SECTION A-A

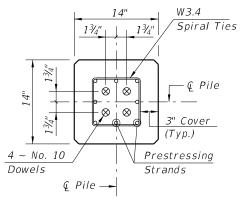
ALTERNATE STRAND PATTERNS

- 8 ~ 0.6" Ø, Grade 270 LRS, at 33 kips
- $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 31 kips
- $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips
- $12 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips
- 16 ~ ¾" Ø, Grade 270 LRS, at 16 kips



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

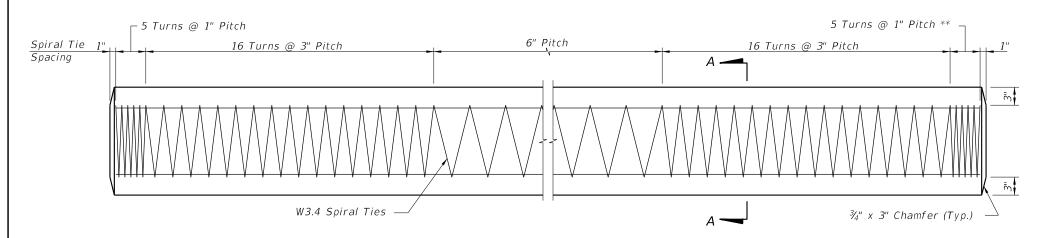
PILE SPLICE REINFORCEMENT DETAILS

NOTES:

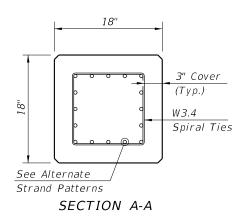
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

 Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips

 $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

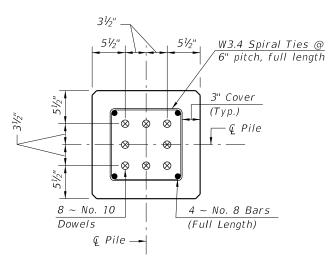
 $16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips

 $20 \sim \frac{7}{16}$ Ø, Grade 270 LRS, at 21 kips

 $24 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 17 kips

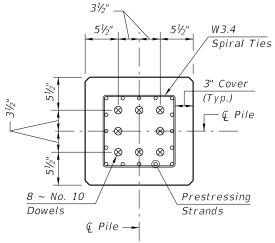
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



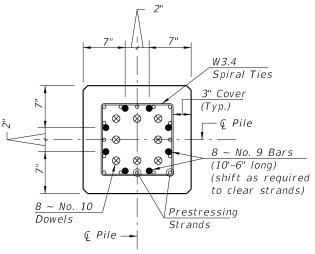
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

REVISION 01/01/12

DESCRIPTION:



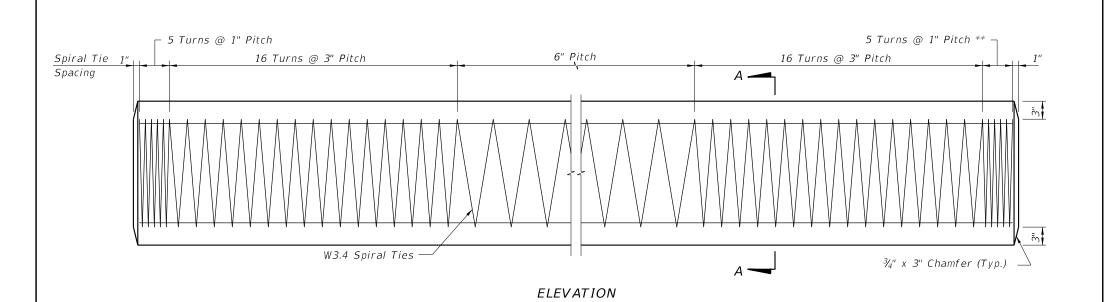
FY 2021-22 STANDARD PLANS

18" SQUARE PRESTRESSED CONCRETE PILE

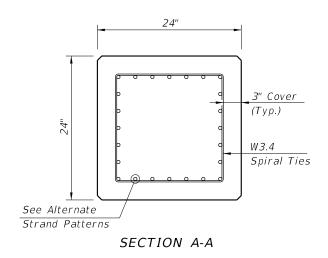
INDEX

SHEET

455-018 1 of 1



** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

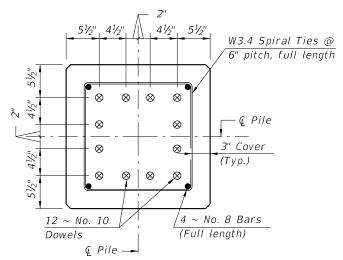
16 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

 $20 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

 $24 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips

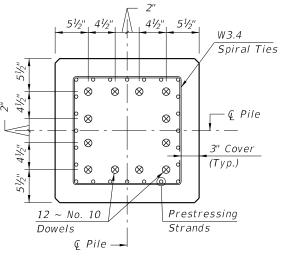
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



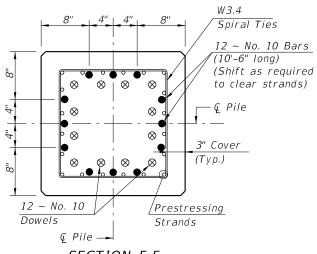
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

REVISION 01/01/12

DESCRIPTION:

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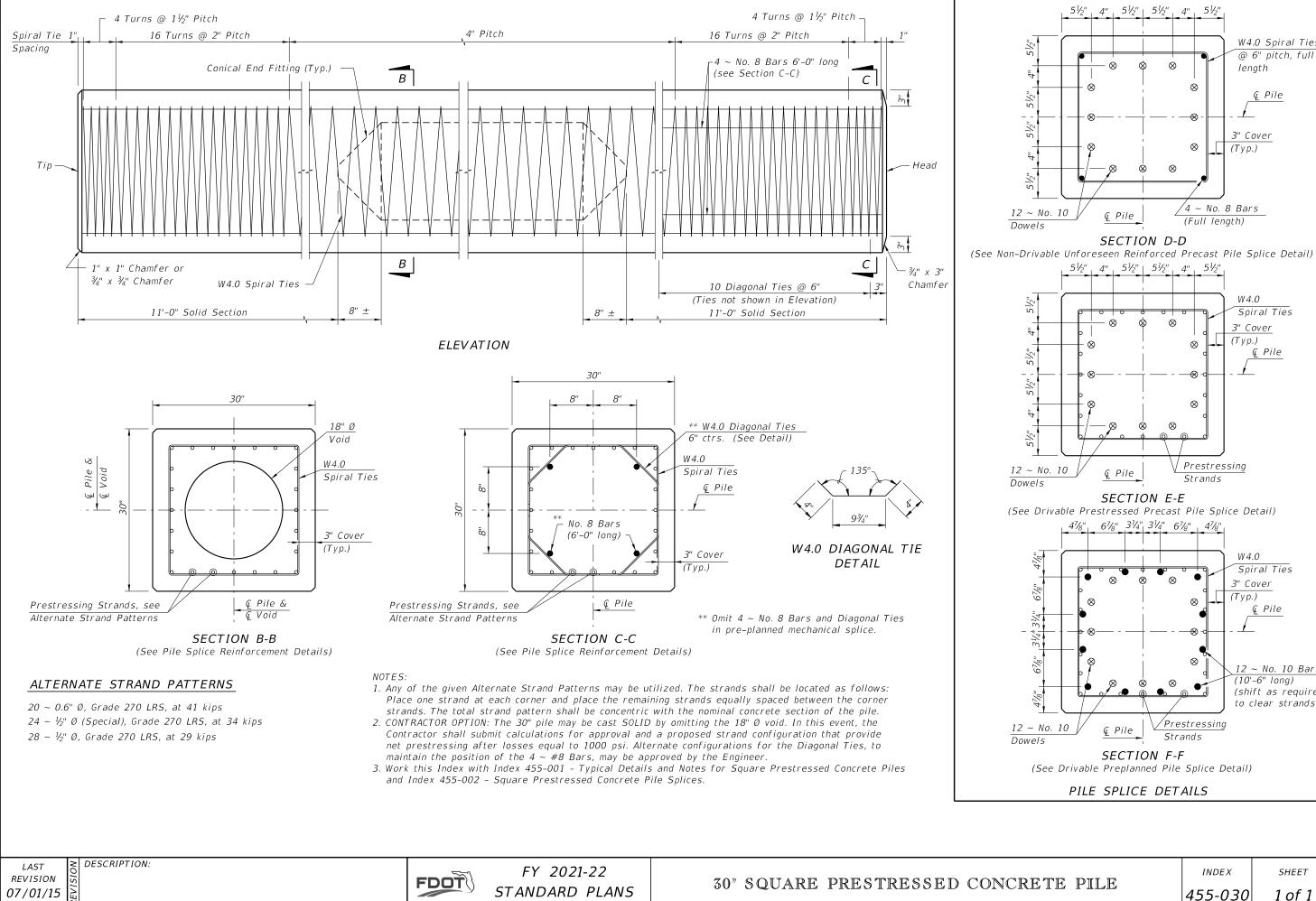
FY 2021-22 STANDARD PLANS

24" SQUARE PRESTRESSED CONCRETE PILE

INDEX

SHEET

455-024 1 of 1



SHEET

W4.0 Spiral Ties

@ 6" pitch, full

€ Pile

length

3" Cover (Typ.)

W4.0

3" Cover (Typ.)

W4.0

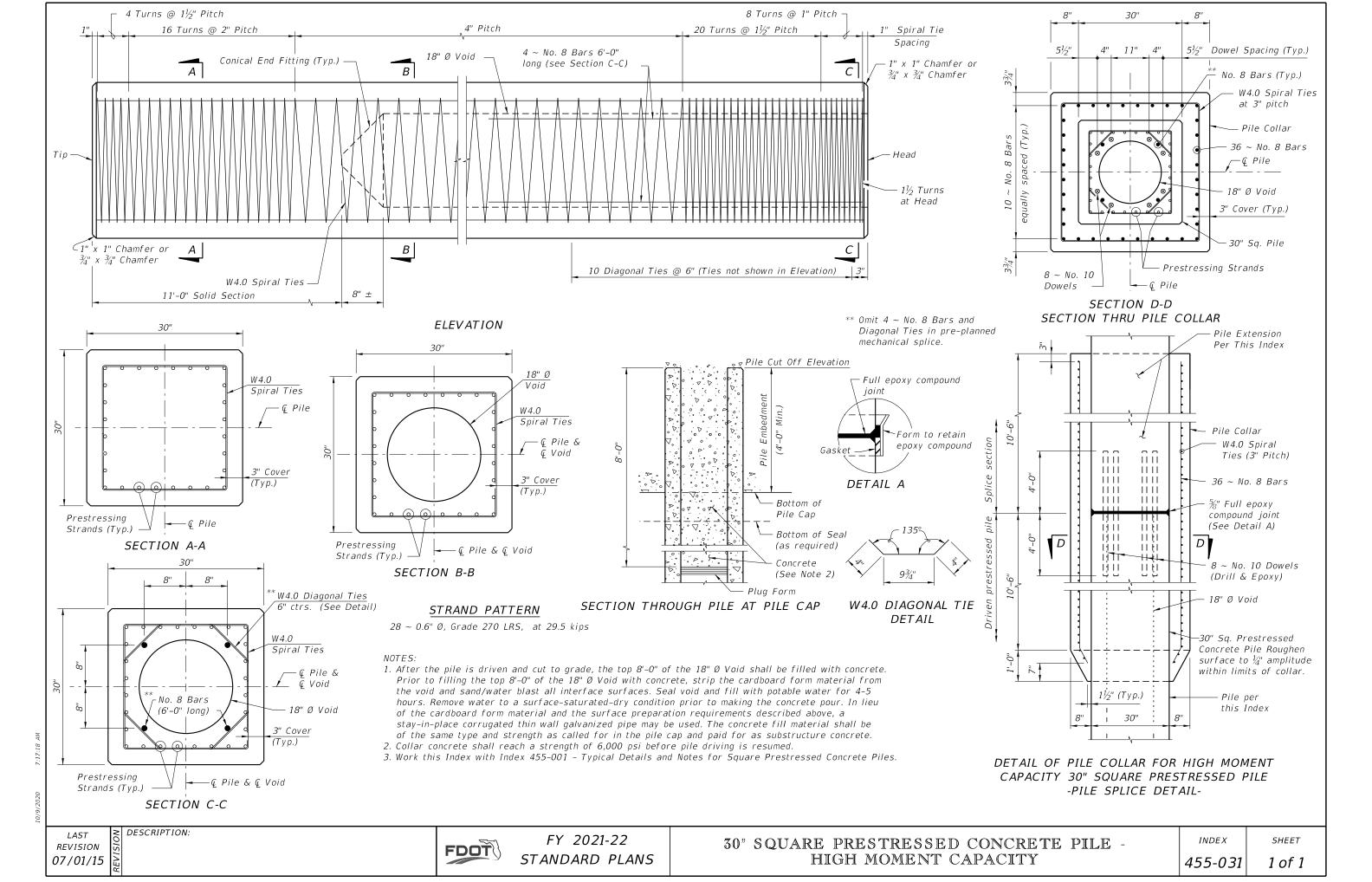
Spiral Ties 3" Cover (Typ.)

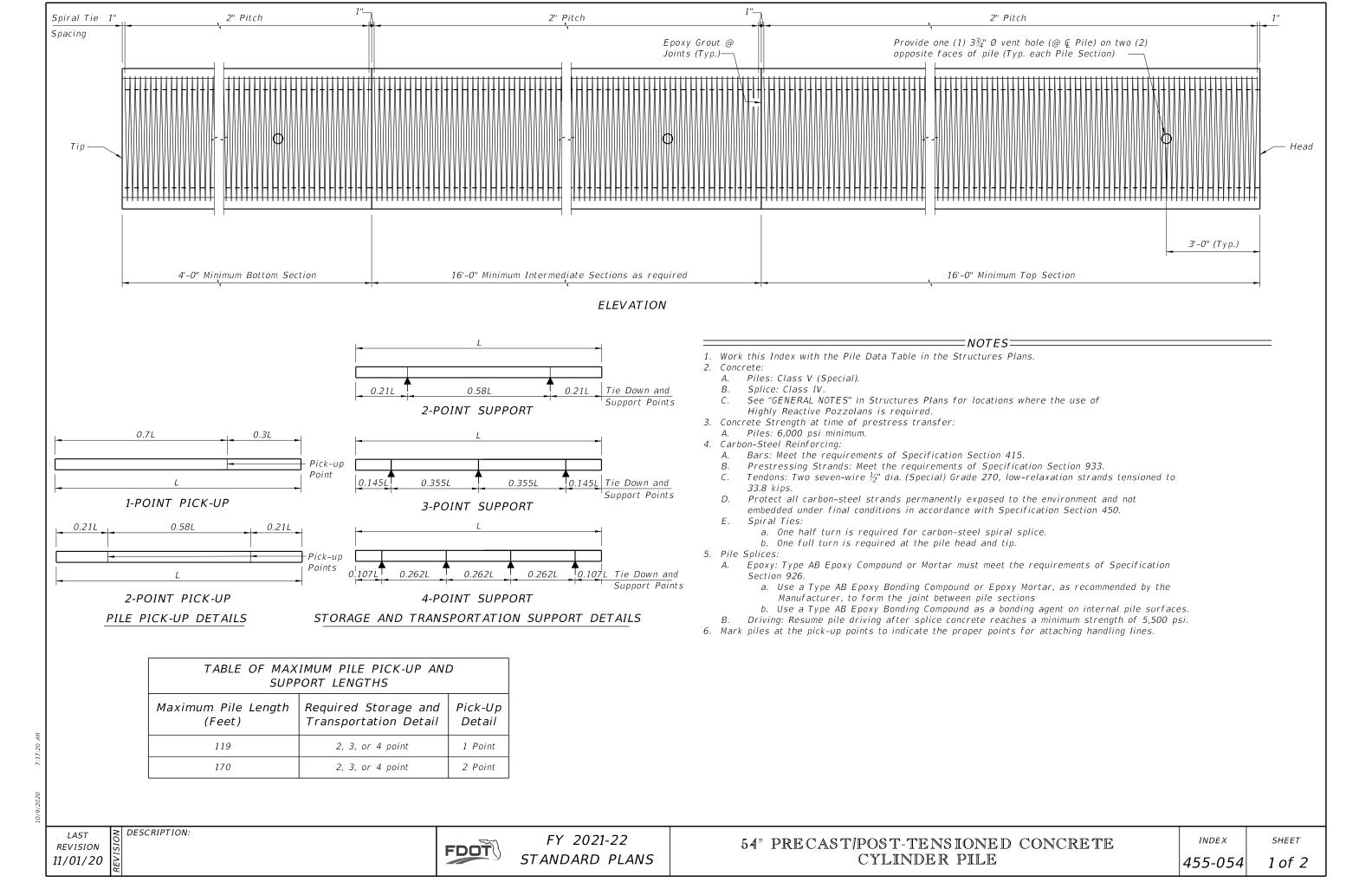
12 ~ No. 10 Bars (10'-6" long)

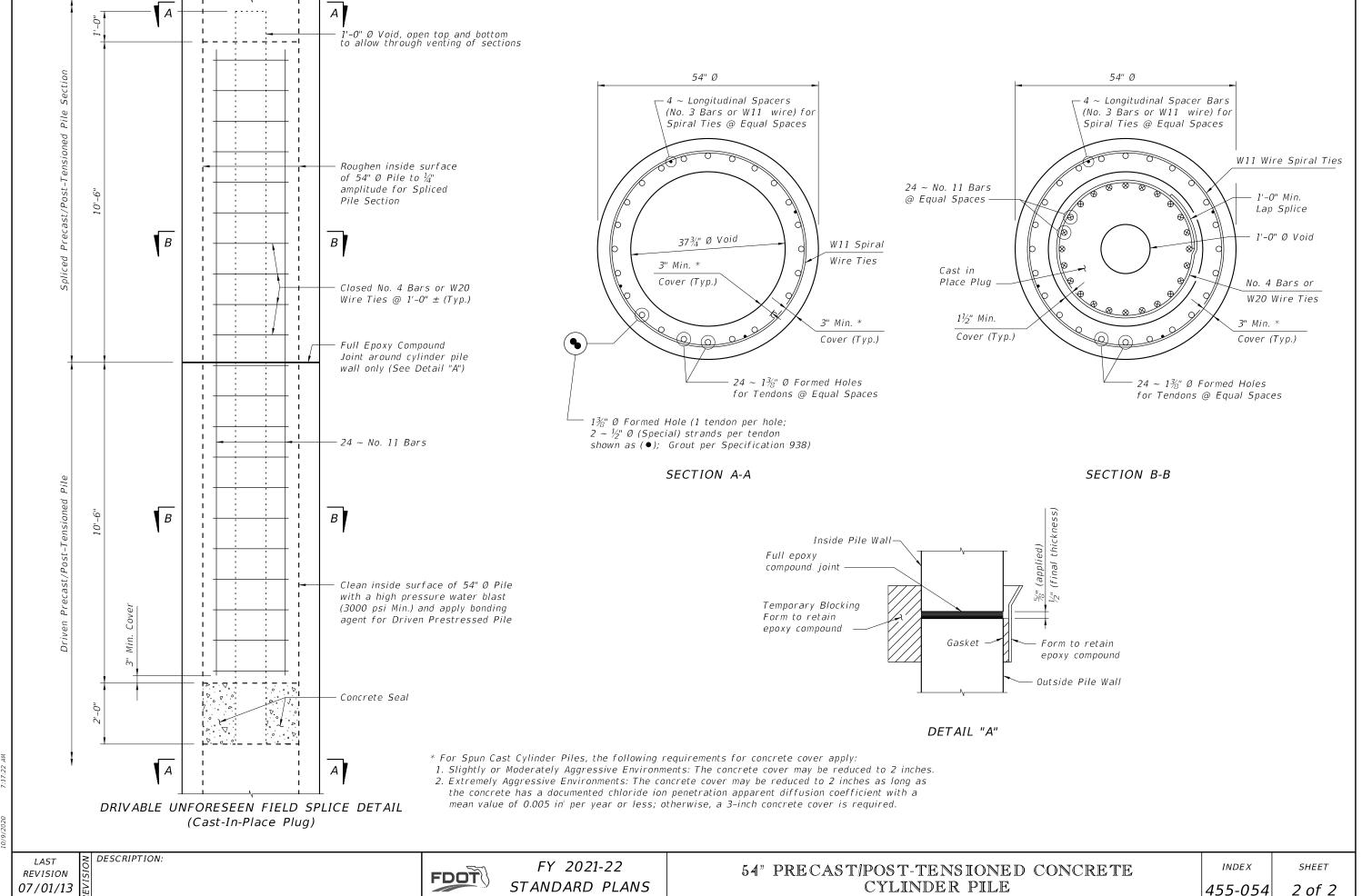
(shift as required

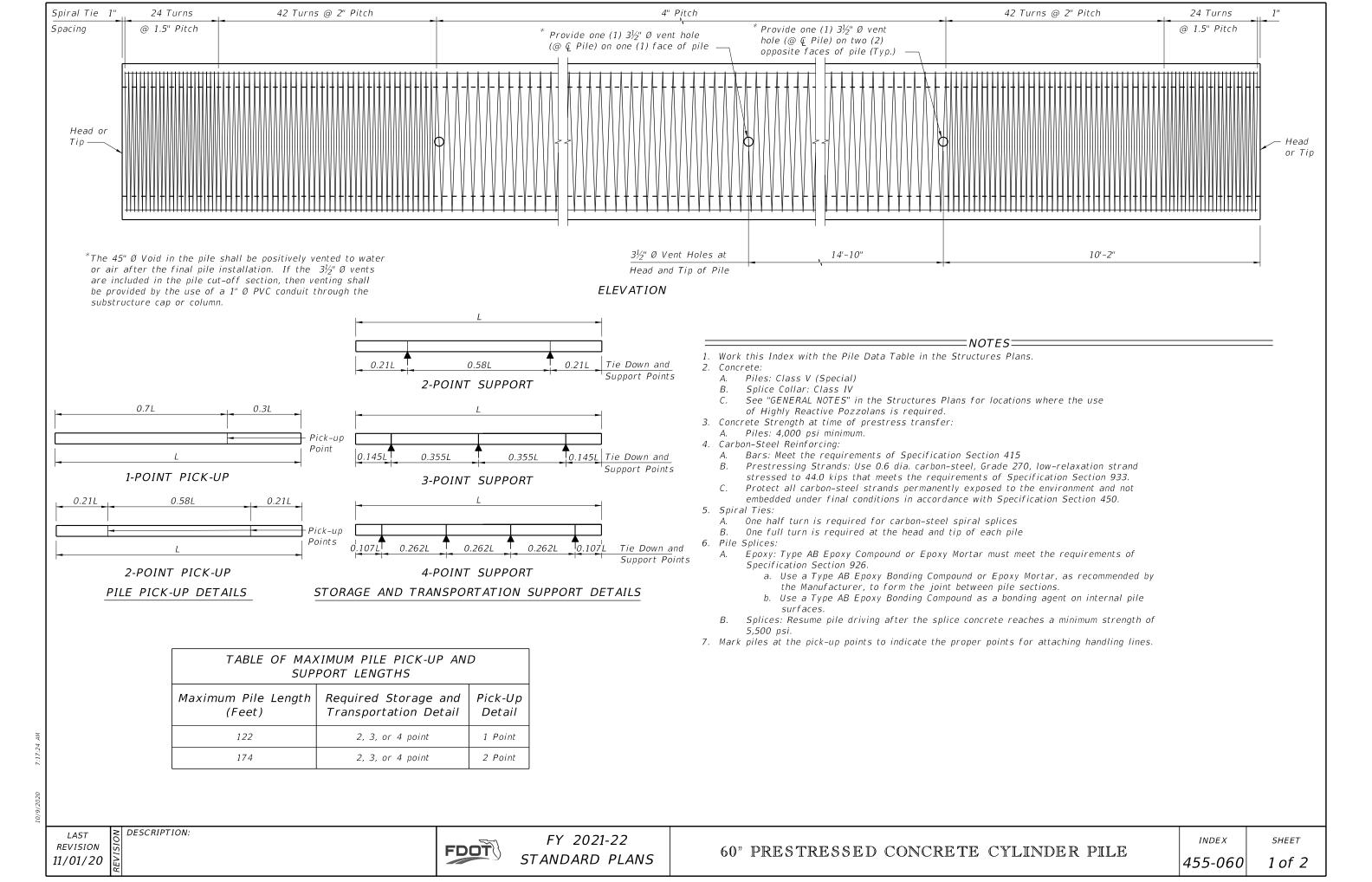
to clear strands)

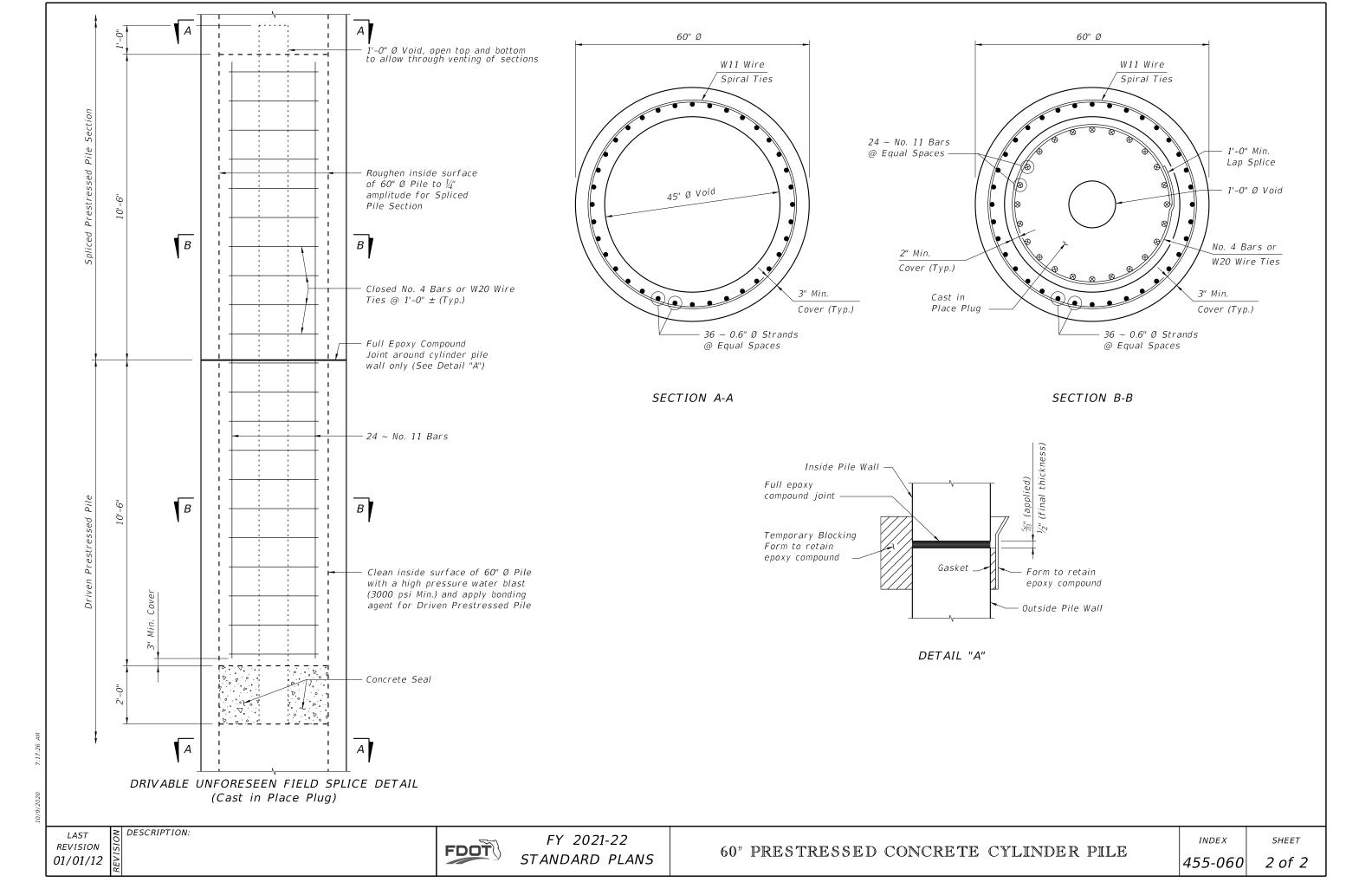
Spiral Ties











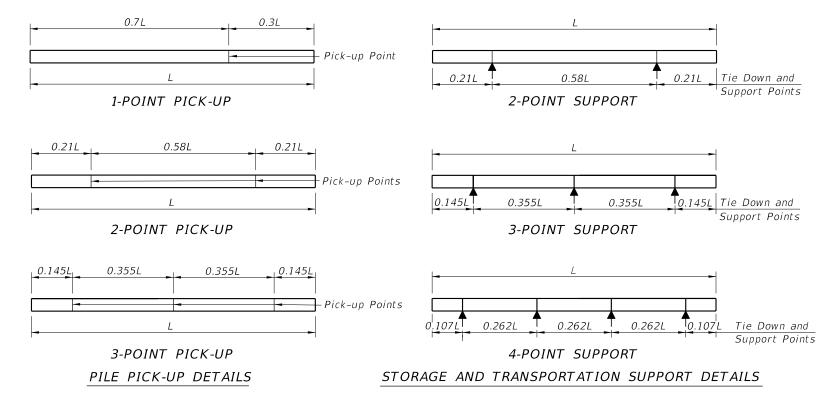
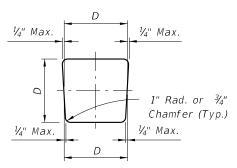
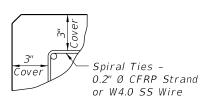


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = S	Square	Pile S	ize (in	ches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point				
	69	75	85	98	124	2, 3, or 4 point	2 Point				
	99	107	121	140	178	3 or 4 point	3 Point				



TYPICAL PILE SHAPE FOR MOLD FORMS



DETAIL SHOWING TYPICAL COVER

PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-102), the Prestressed Concrete Pile Standards (Index 455-112, 455-114, 455-118, 455-124, 455-130, and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V (Special)
 - . See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required for options using stainless steel strand and reinforcing.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
- 4. Reinforcing:
 - A. Bars:
 - a. Stainless Steel: Meet the requirements of Specification Section 931 for Type 304, Grade 75.
 - b. Carbon FRP: Meet the requirements of Specification Section 932.
 - B. Prestressing Strands:
 - a. Stainless Steel: Seven-wire HSSS, Grade 240 strand, meeting the requirements of Specification Section 933.
 - b. Carbon FRP: Meet the requirements of Specification Section 933.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
 - B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

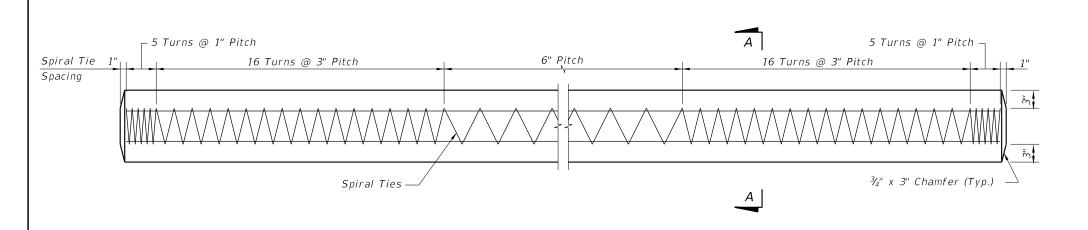
CONCRETE PILE SPLICES

455-102

1 of 1

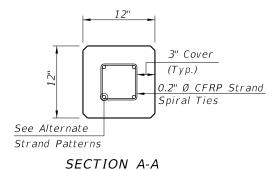
STANDARD PLANS

01/01/16



ALTERNATE STRAND PATTERNS

4 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips $4 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 41 kips

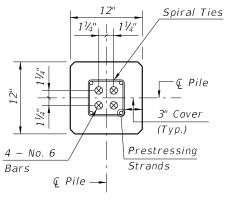


6" pitch, full length 11/4" | 11/4" __ € Pile 3" Cover (Typ.)4 ~ No. 6 Bars 4 ~ No. 6 Bars (Full Length) ¢ Pile →

Spiral Ties @

SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized.

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

DESCRIPTION:

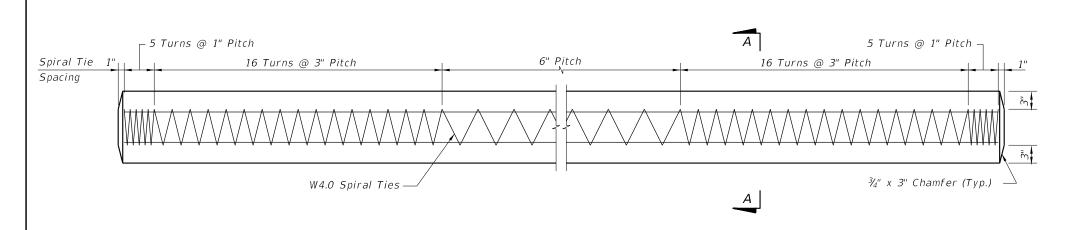
FDOT

FY 2021-22 STANDARD PLANS

12" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

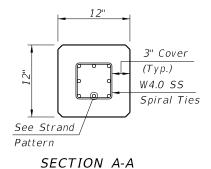
INDEX 455-112

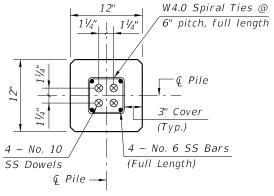
SHEET 1 of 2



STRAND PATTERN

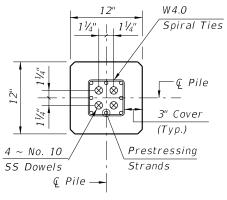
 $8 \sim \frac{1}{2}$ " Ø, HSSS at 24 kips 8 ~ 0.6" Ø, HSSS at 26 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

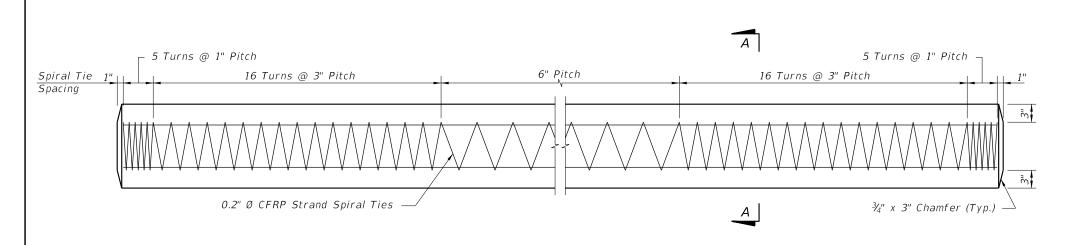
DESCRIPTION:

FDOT

FY 2021-22 STANDARD PLANS

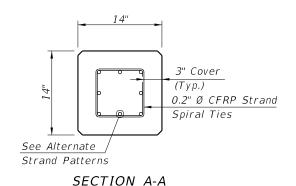
12" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX



ALTERNATE STRAND PATTERNS

8 ~ 0.6" Ø, CFRP 7-Strand, at 31.5 kips $8 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 30.5 kips



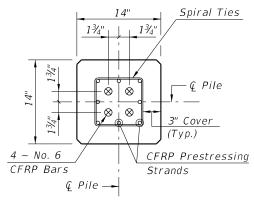
3" Cover (Typ.)4 ~ No. 6 4 ~ No. 6 CFRP Bars CFRP Bars (Full Length) ¢ Pile →

13/4"

Spiral Ties @ 6" pitch, full length

SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

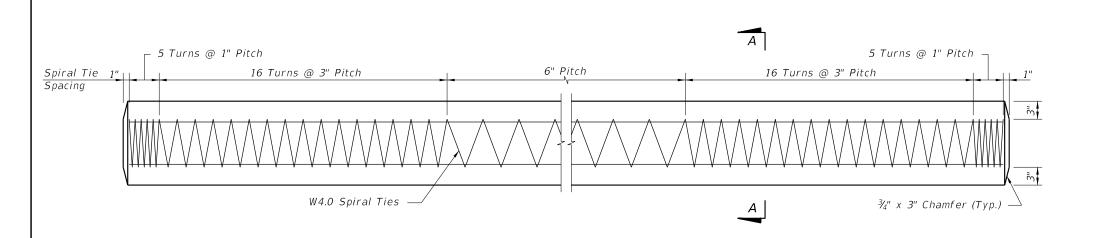
Place one strand at each corner and equally space the remaining strands between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

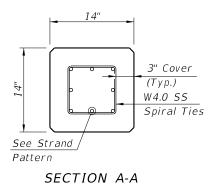
DESCRIPTION: FDOT STANDARD PLANS 455-114

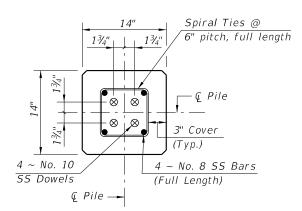


ELEVATION

STRAND PATTERN

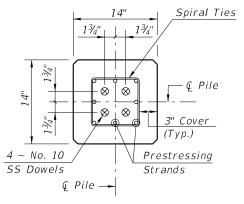
 $12 \sim \frac{1}{2}$ " Ø, HSSS at 23 kips 8 ~ 0.6" Ø, HSSS at 35 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

FDOT

STANDARD PLANS

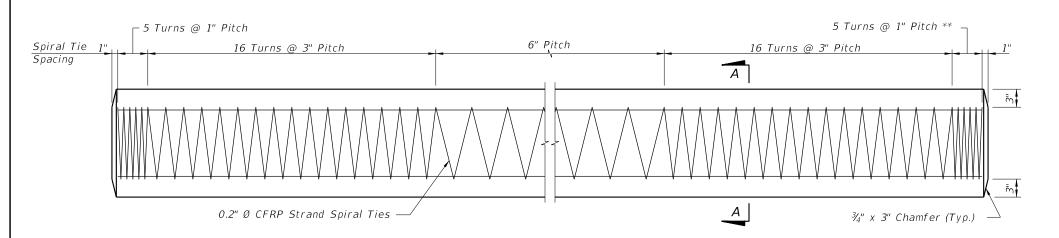
14" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

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SHEET 2 of 2

DESCRIPTION:

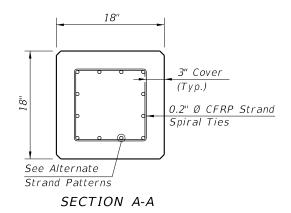
FY 2021-22



** See Note 4 on Index 455-102

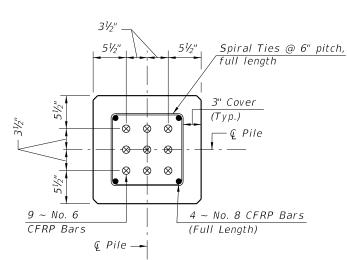
ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, CFRP 7-Strand, at 34 kips $12 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 33 kips



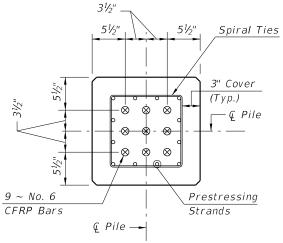
NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



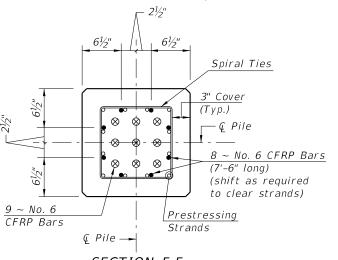
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

CFRP PRESTRESSED PILE DETAILS

11/01/16

DESCRIPTION:

FDOT

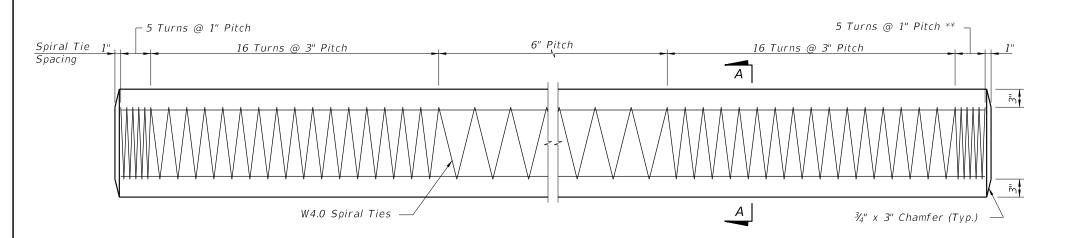
FY 2021-22 STANDARD PLANS

18" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX 455-118

SHEET

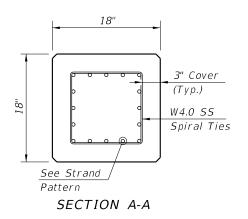
REVISION



** See Note 4 on Index 455-102

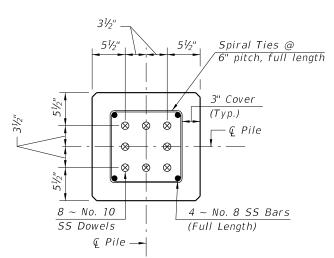
STRAND PATTERN

 $16 \sim \frac{1}{2}$ " Ø, HSSS, at 26 kips 12 ~ 0.6" Ø, HSSS, at 35 kips



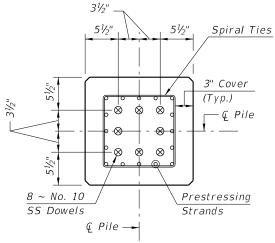
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



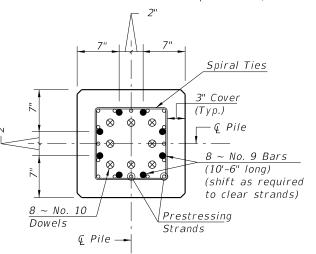
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Predtresses Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

DESCRIPTION:

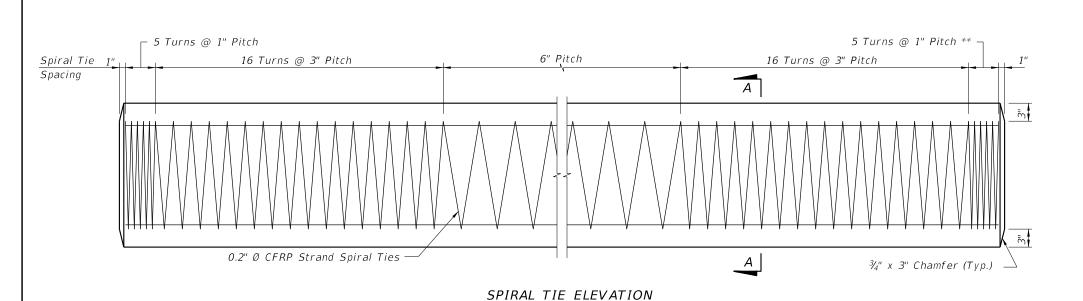
FDOT

FY 2021-22 STANDARD PLANS 18" SQUARE CFRP & SS PRESTRESSED

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SHEET

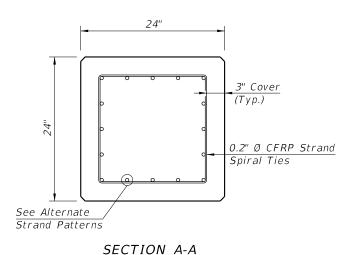
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** See Note 4 on Index 455-102

ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips $20 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 35 kips



- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal

6" Pitch, Full Length \otimes \otimes - @ Pile \otimes \otimes \otimes 3" Cover (Typ.)/4 ~ No. 6 CFRP Bars 16 ~ No. 6 CFRP Bars (Full length) Ç Pile → SECTION D-D (See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail) 6¾" | 3½" | | | 3½" | 6¾" | Spiral Ties \otimes \otimes · @ Pile \otimes \otimes \otimes \otimes 3" Cover (Typ.) \otimes /Prestressing 16 ~ No. 6 Strands CFRP Bars Ç Pile → SECTION E-E (See Drivable Prestressed Precast Pile Splice Detail) 63/4" | 31/2" | , 31/2", 63/4" Spiral Ties 12 ~ No. 6 CFRP Bars (8'-6" long) (Shift as required to clear strands) — ∉ Pile 3" Cover (Typ.)Prestressing 16 ~ No. 6 Strands CFRP Bars € Pile → SECTION F-F (See Drivable Preplanned Prestressed Precast Pile Splice Detail) CFRP PILE SPLICE REINFORCEMENT DETAILS

 $6\frac{3}{4}$ " $3\frac{1}{2}$ " $\left| \right| \left| 3\frac{1}{2}\right| 6\frac{3}{4}$ "

Spiral Ties

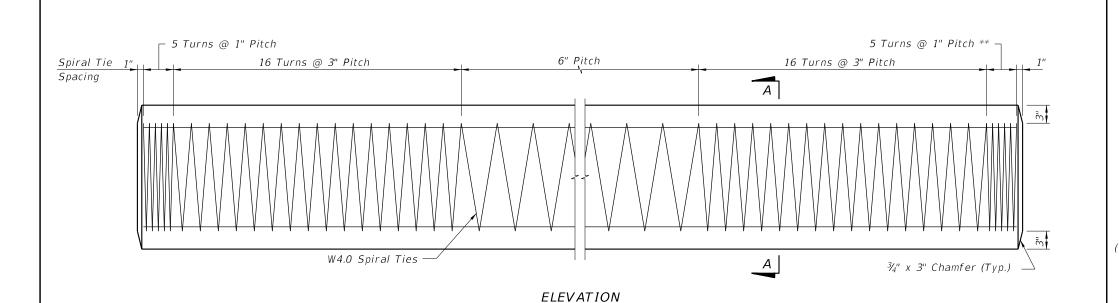
CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/20

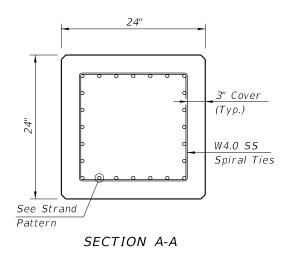
FDOT

FY 2021-22 STANDARD PLANS

concrete section of the pile.



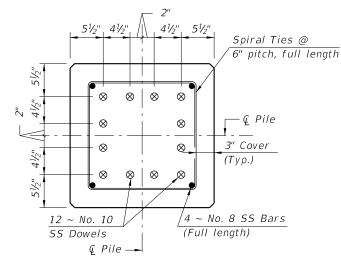
** See Note 4 on Index 455-102



STRAND PATTERN

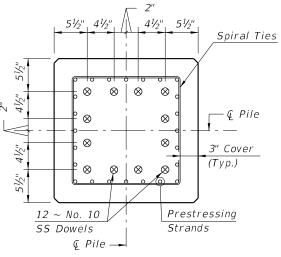
 $28 \sim \frac{1}{2}$ " Ø, HSSS at 26 kips 20 ~ 0.6" Ø, HSSS at 35 kips

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal



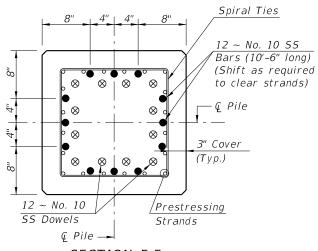
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

DESCRIPTION:

FDOT

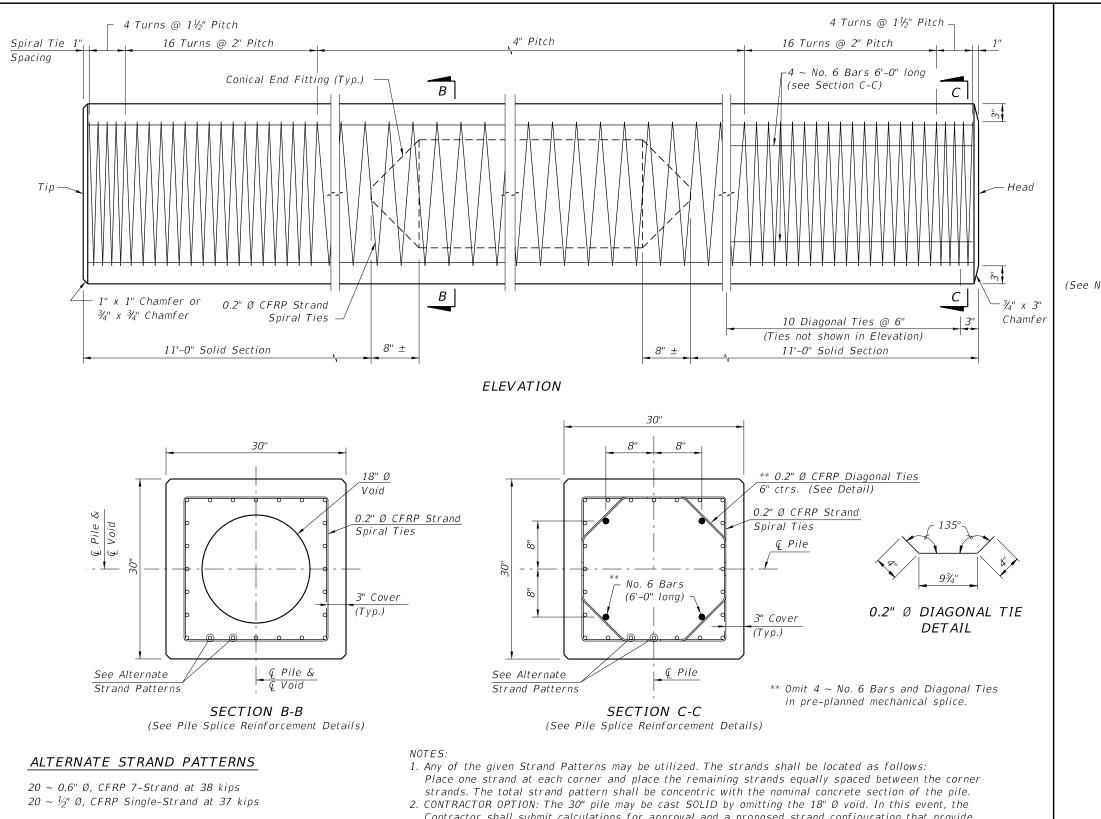
FY 2021-22

concrete section of the pile.

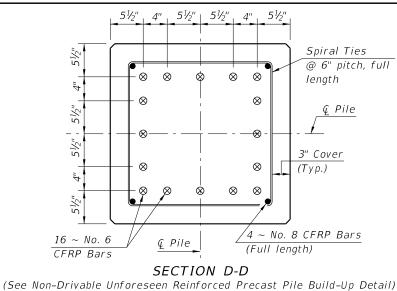
24" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

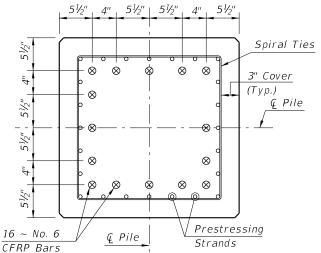
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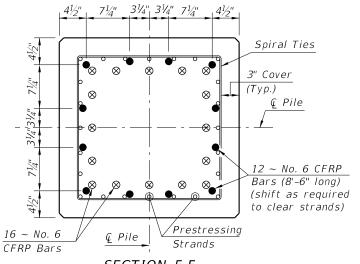
- Contractor shall submit calculations for approval and a proposed strand configuration that provide net prestressing after losses equal to 1000 psi. Alternate configurations for the Diagonal Ties, to maintain the position of the $4\sim$ #6 Bars, may be approved by the Engineer.
- 3. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.





SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE DETAILS

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

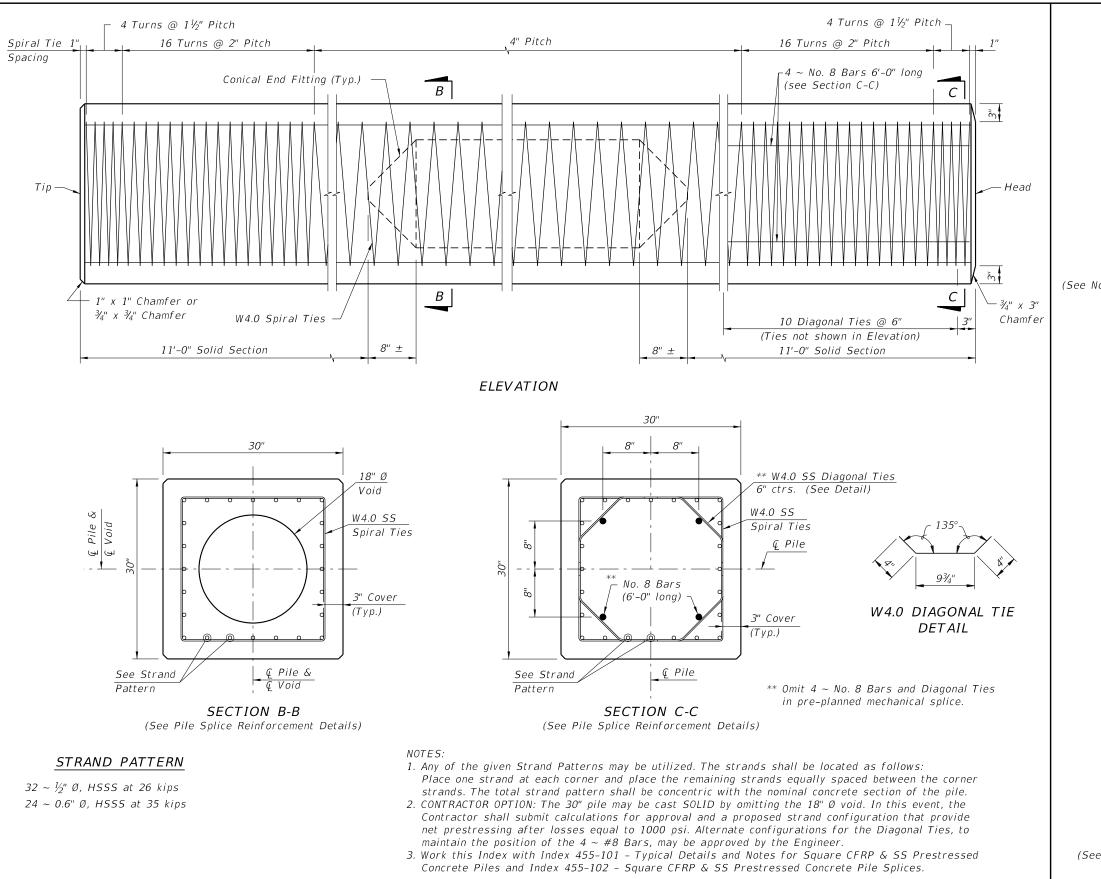
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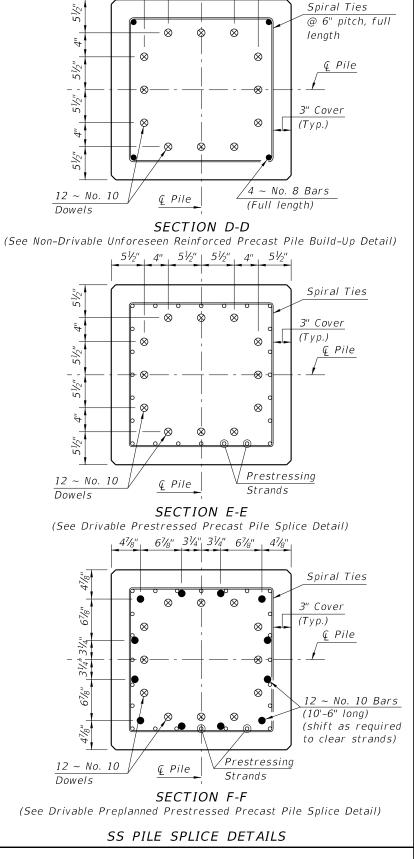
FDOT

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CONCRETE PILE

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 $5\frac{1}{2}$ " | 4" | $5\frac{1}{2}$ " | $5\frac{1}{2}$ " | 4" | $5\frac{1}{2}$ "

SS PRESTRESSED PILE DETAILS

REVISION 11/01/20

DESCRIPTION:

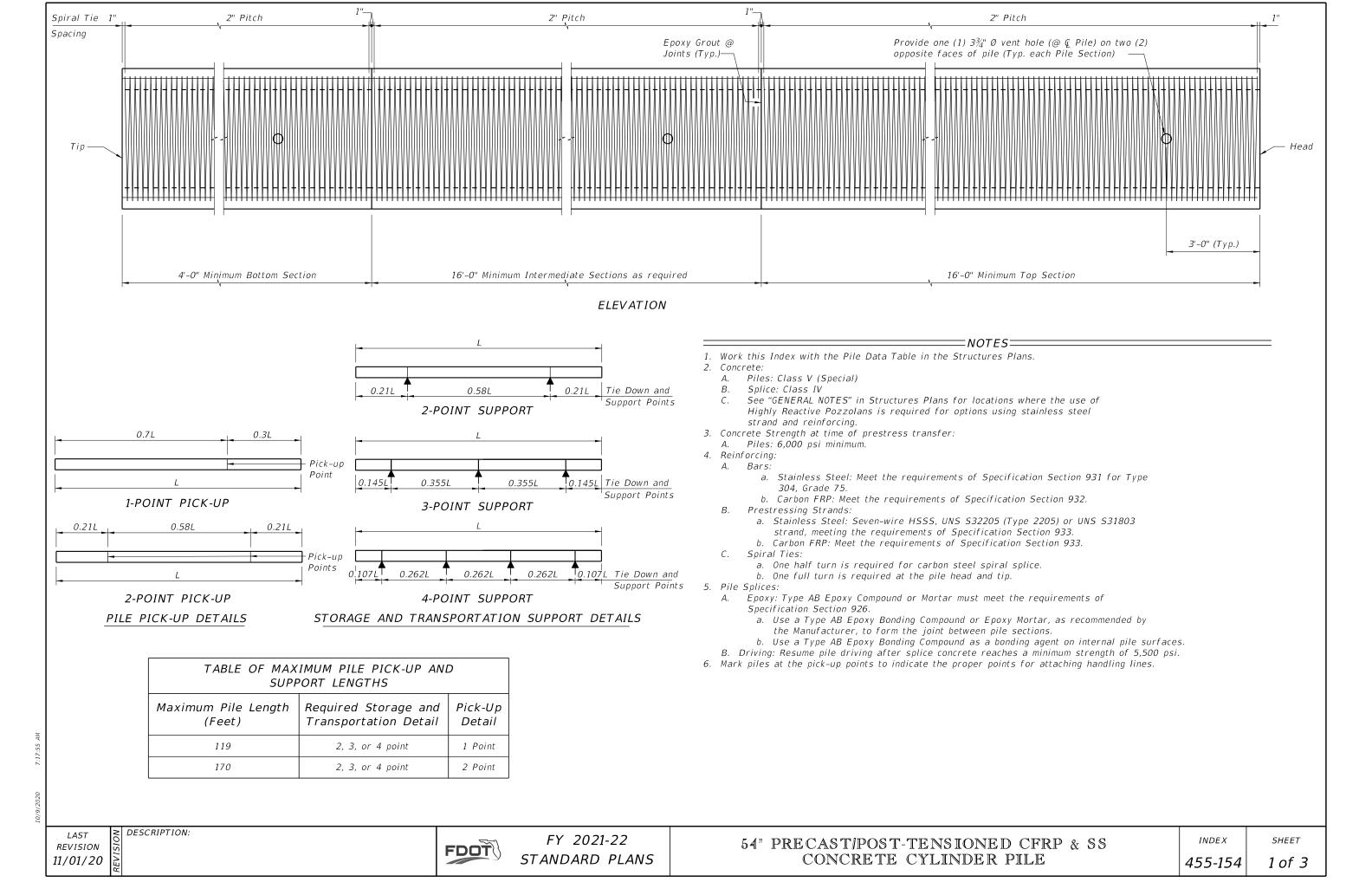
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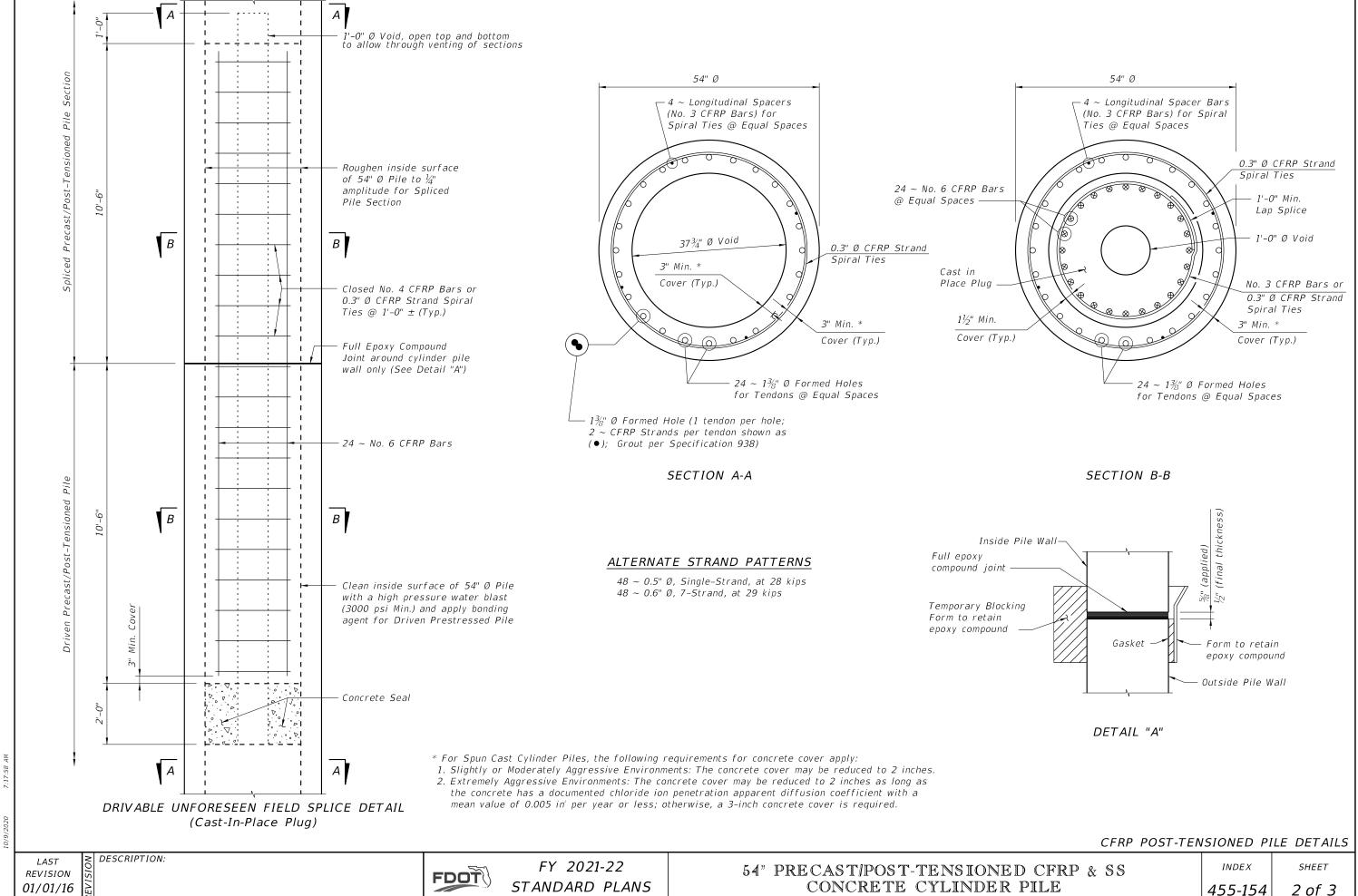
FY 2021-22 STANDARD PLANS

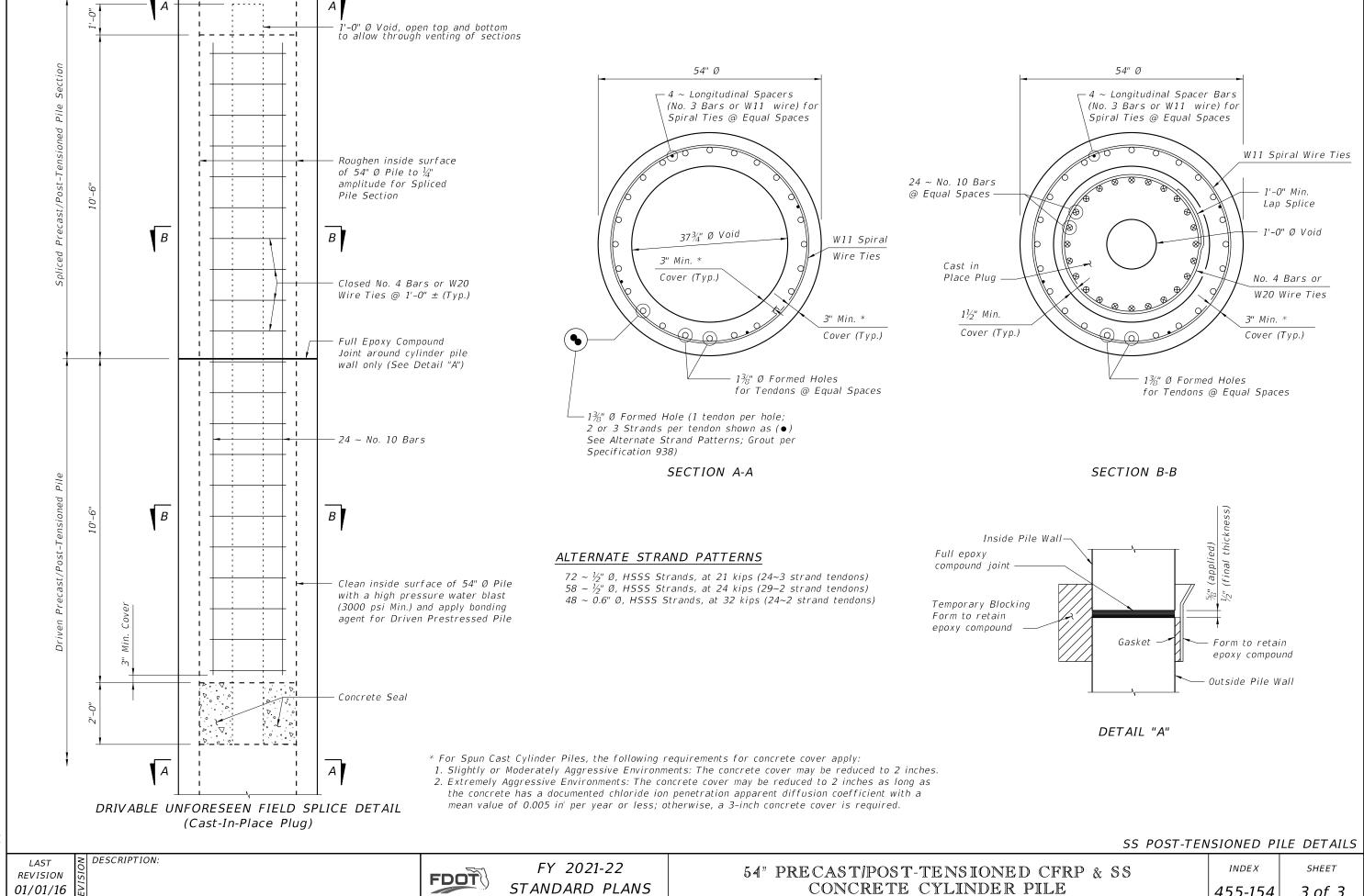
30" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

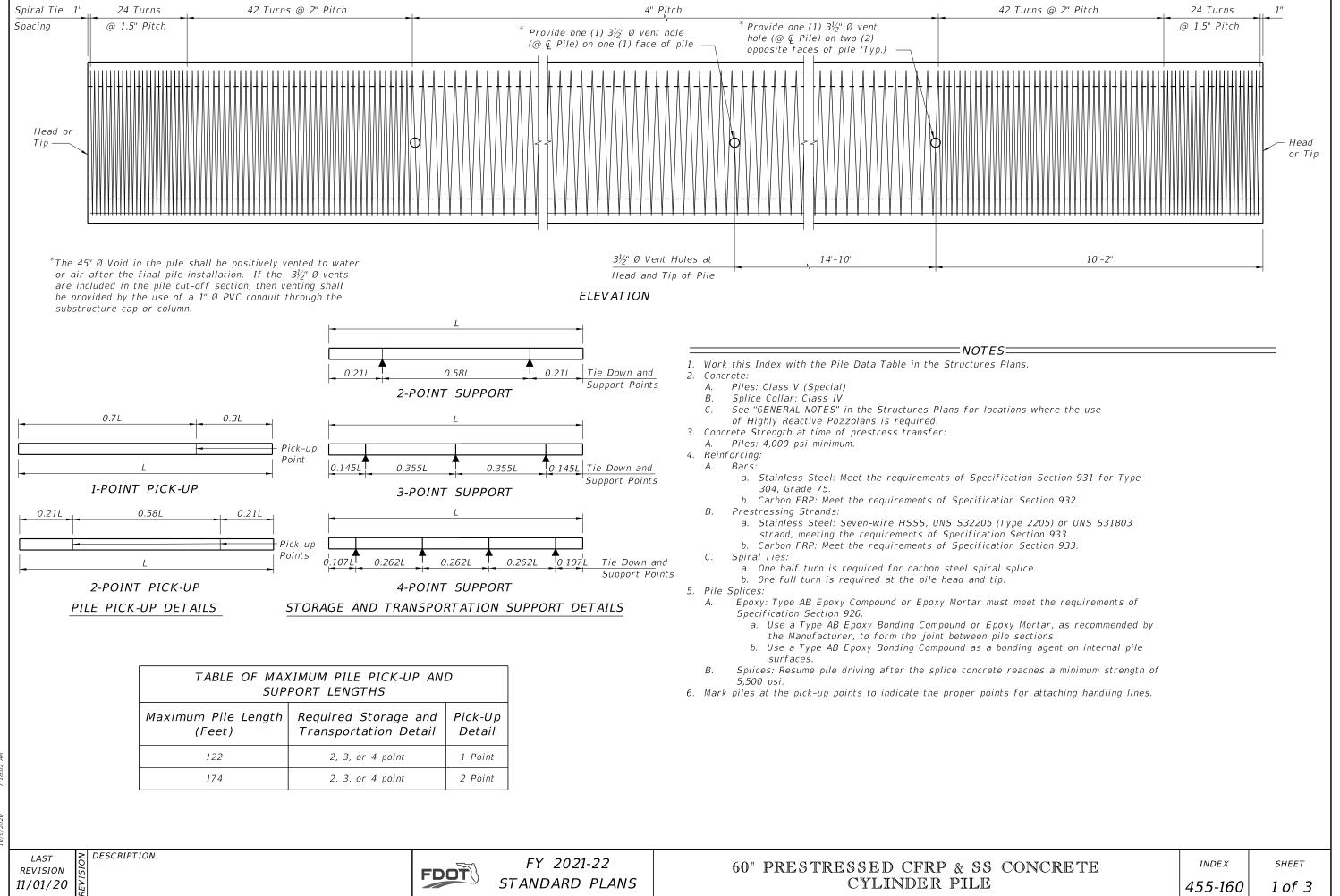
INDEX 455-130 SHEET

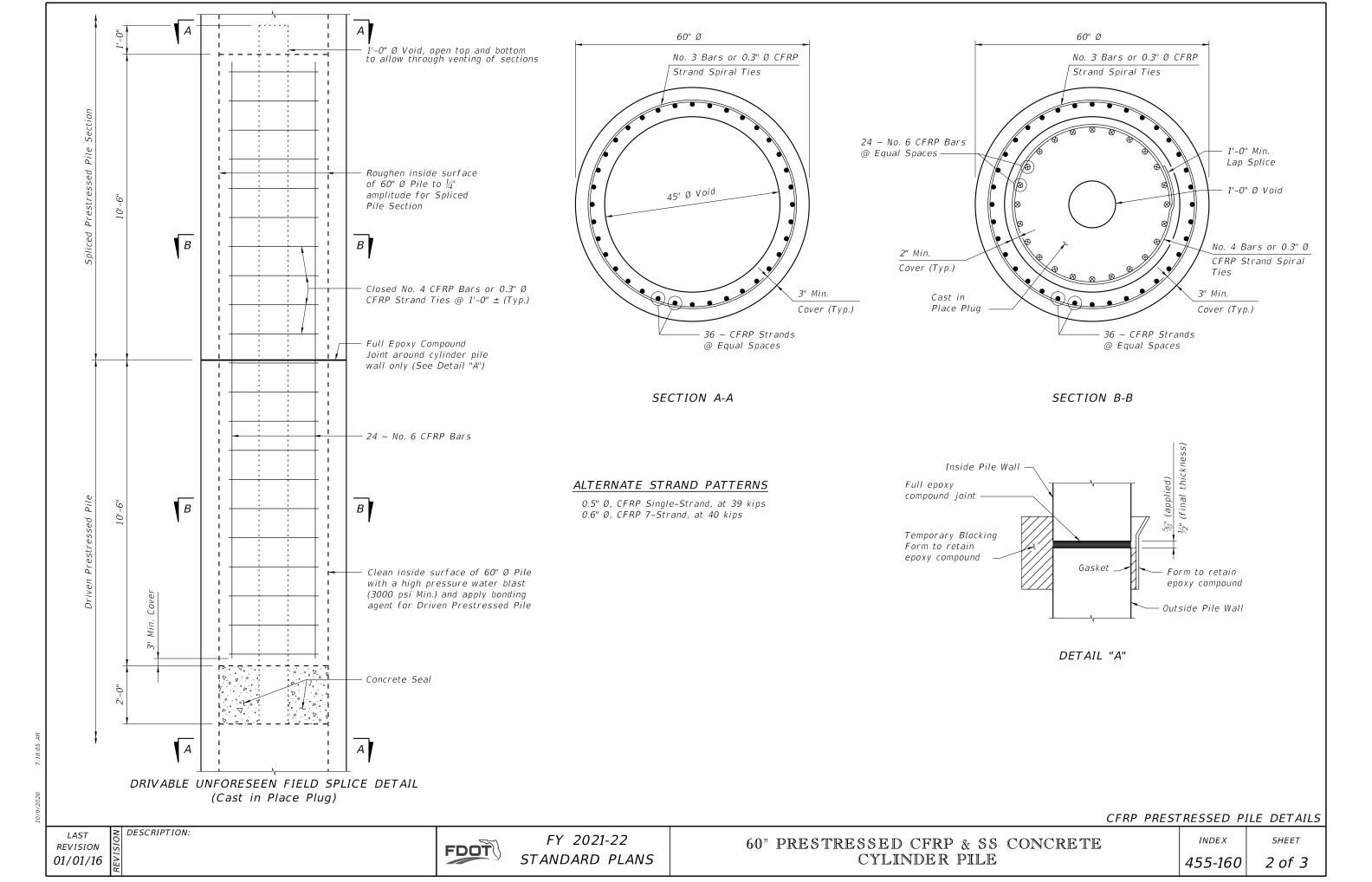
2 of 2

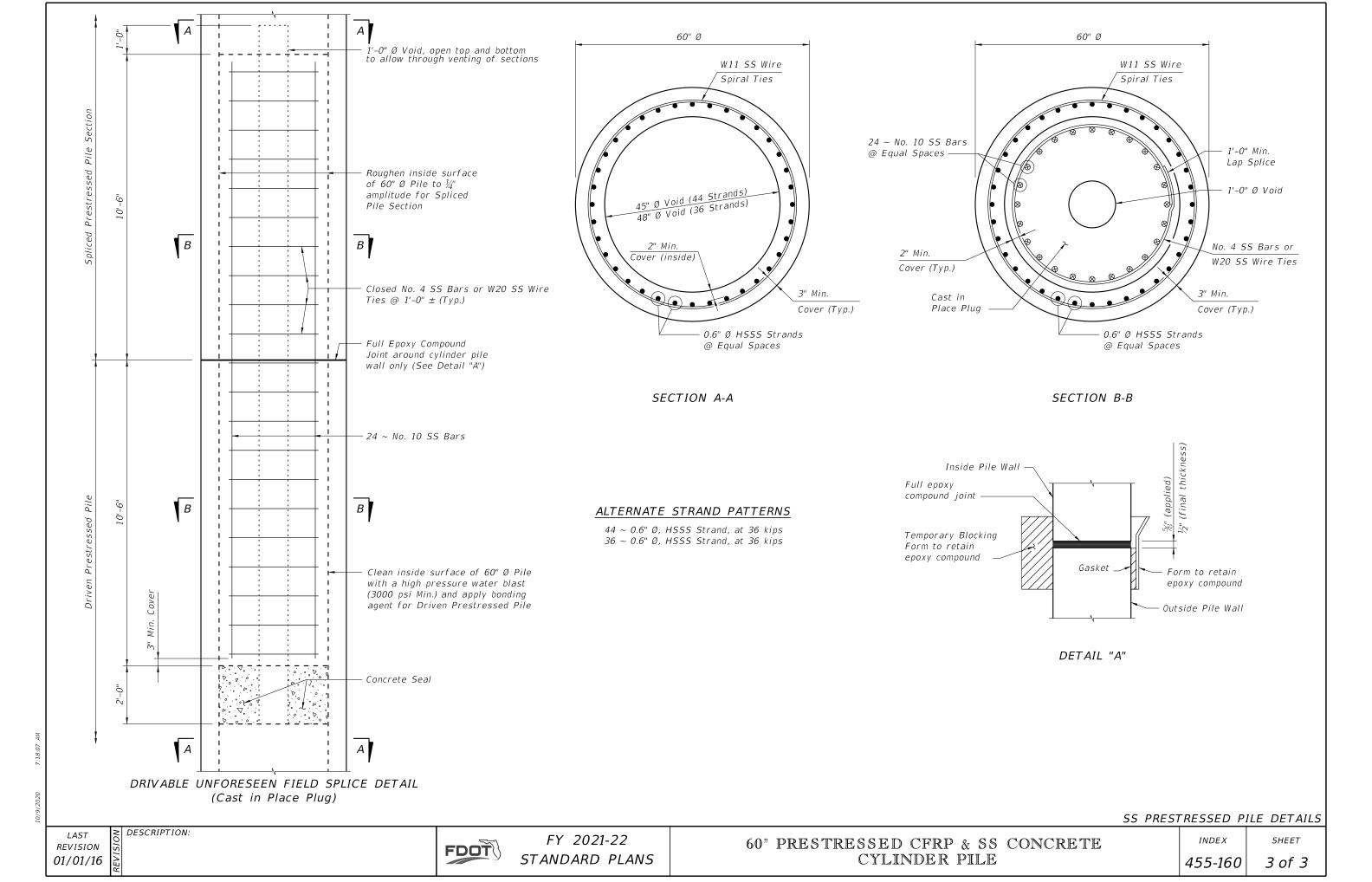


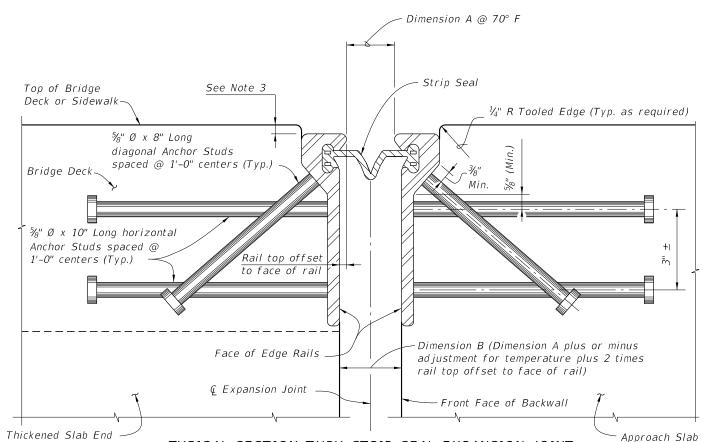




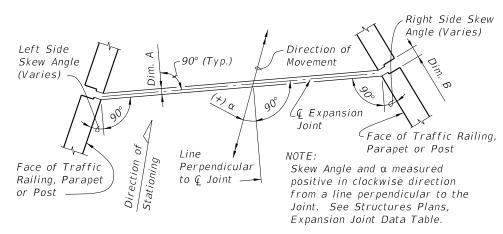




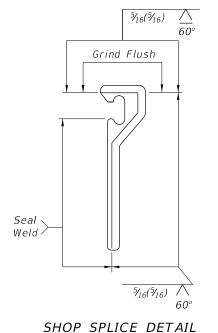




TYPICAL SECTION THRU STRIP SEAL EXPANSION JOINT (Begin or End Concrete Girder Bridge shown, Intermediate Supports and Steel Girder Bridge similar. Reinforcing Steel and Girder details not shown for clarity.)



MOVEMENT SCHEMATIC



GENERAL NOTES:

- 1. Furnish Strip Seal Expansion Joint Systems in accordance with Specification Section 458.
- 2. Shape of Edge Rail shown is representative, minor variations depending on manufacturer
- 3. Recess the Edge Rail below the concrete surface in accordance with Specification Section 458.
- 4. Refer to the Expansion Joint Data Table in the Structures Plans for joint movement and Dimension A.
- 5. Refer to Specification Section 458 for installation and fabrication requirements.

REVISION 11/01/19

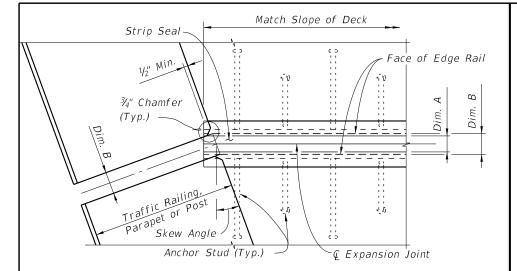
DESCRIPTION:

FY 2021-22 STANDARD PLANS EXPANSION JOINT SYSTEM -STRIP SEAL

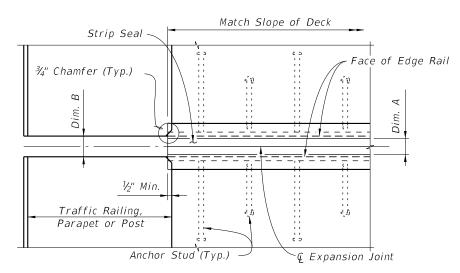
INDEX

SHEET

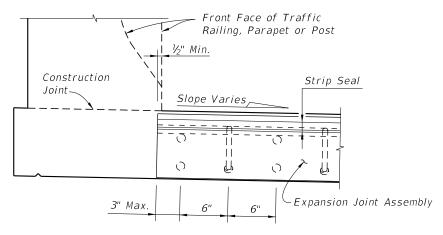
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PARTIAL PLAN VIEW OF SKEWED JOINTS

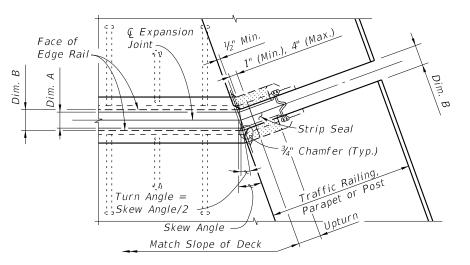


PARTIAL PLAN VIEW OF NONSKEWED JOINTS

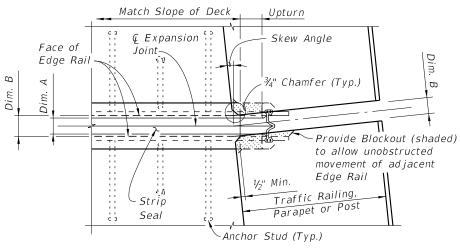


PARTIAL SECTION ALONG G JOINT

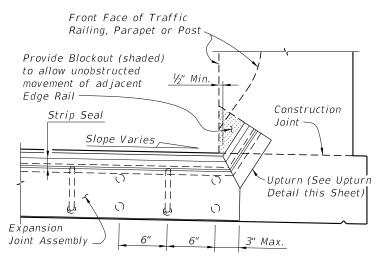
JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPE ≥ 1% (Sidewalk Cover Plate where applicable not shown for clarity)



PARTIAL PLAN VIEW OF JOINTS SKEWED GREATER THAN 6°

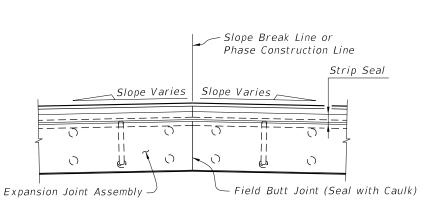


PARTIAL PLAN VIEW OF NONSKEWED JOINTS & JOINTS SKEWED 6° OR LESS

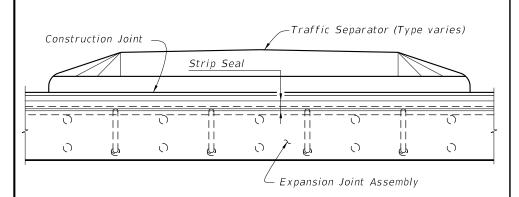


PARTIAL SECTION ALONG Q JOINT

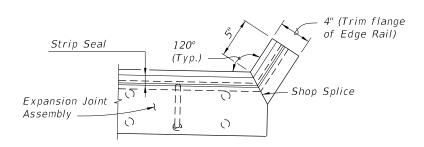
JOINT TREATMENT AT LOW SIDE OF DECK & HIGH SIDE OF DECK WITH SLOPE < 1% (Sidewalk Cover Plate where applicable not shown for clarity)



PARTIAL SECTION ALONG G JOINT AT FIELD BUTT JOINT LOCATION (CROWNED DECK OR SLAB SHOWN)



PARTIAL SECTION ALONG Q JOINT THRU TRAFFIC SEPARATOR



UPTURN DETAIL (TYPICAL AT TRAFFIC BARRIERS AND PARAPETS)

REVISION 11/01/19

DESCRIPTION:

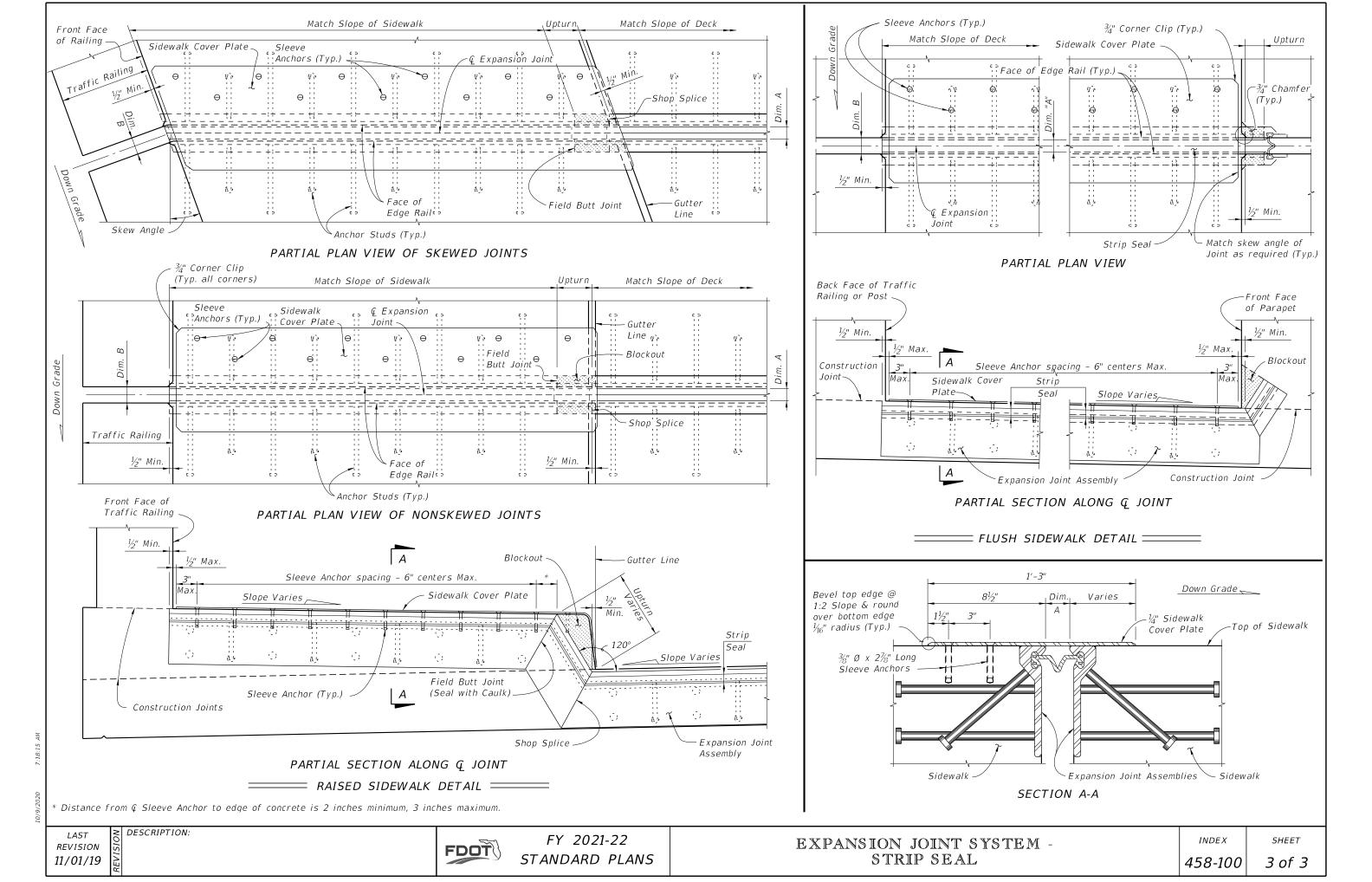
FDOT

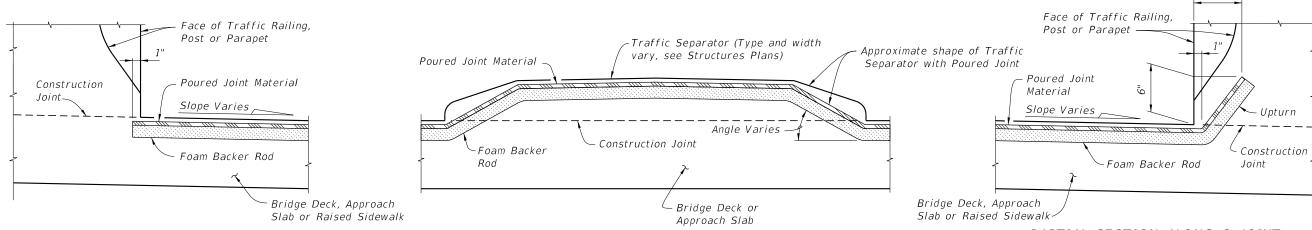
FY 2021-22 STANDARD PLANS EXPANSION JOINT SYSTEM -STRIP SEAL

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SHEET

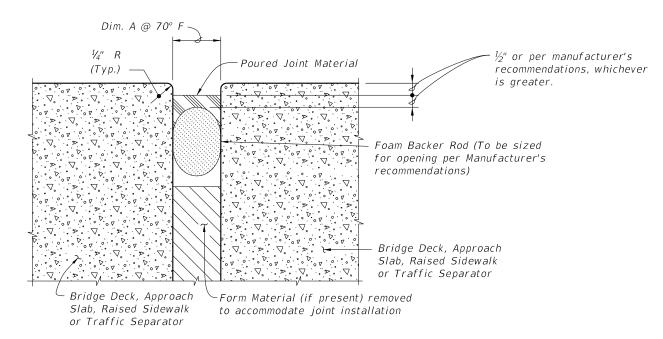
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PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPES 1% OR GREATER

PARTIAL SECTION ALONG Q JOINT, JOINT TREATMENT AT TRAFFIC SEPARATOR PARTIAL SECTION ALONG Q JOINT
JOINT TREATMENT AT LOW SIDE OF DECK OR
HIGH SIDE OF DECK WITH SLOPES < 1%



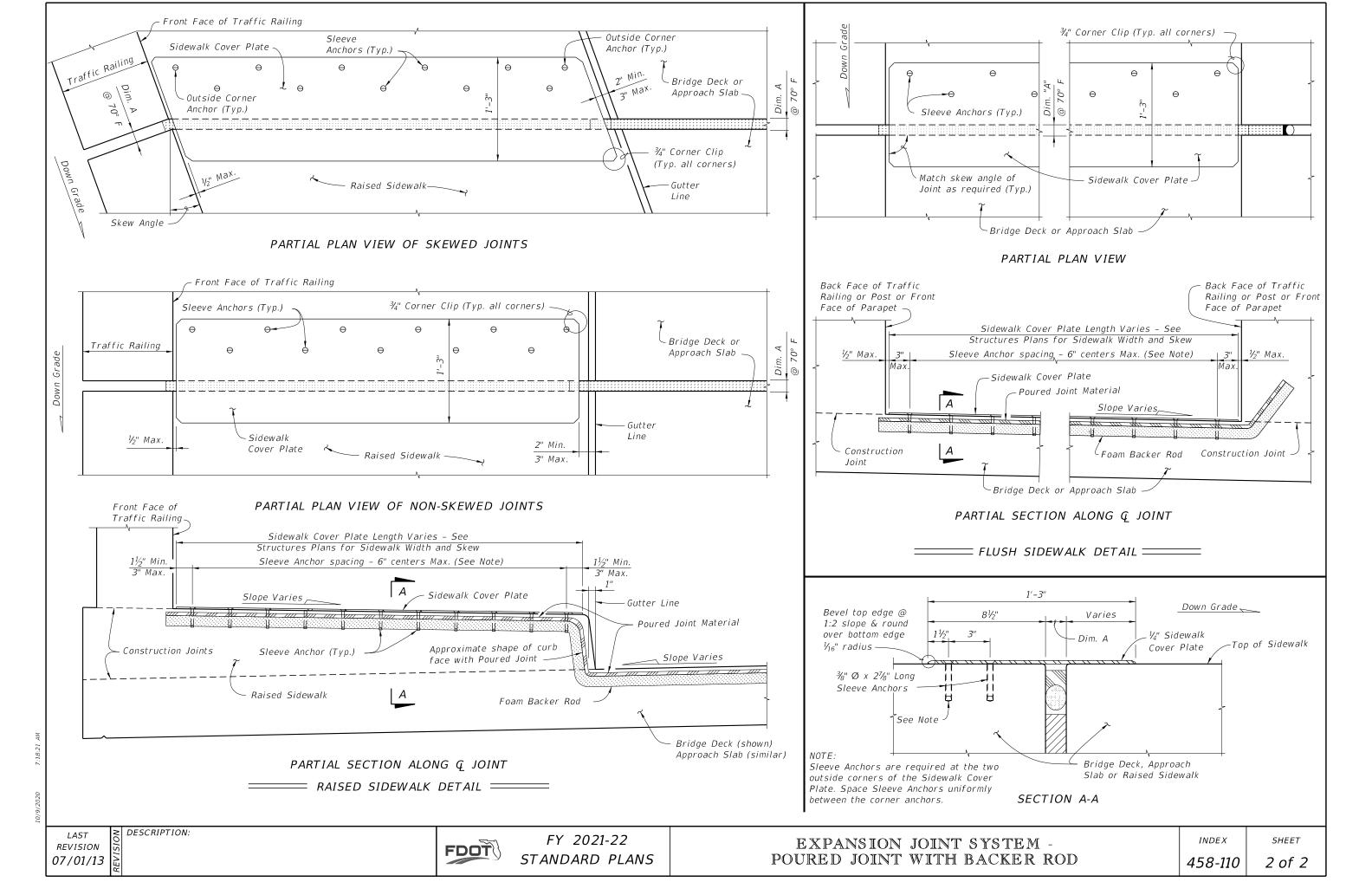
TYPICAL SECTION THRU JOINT

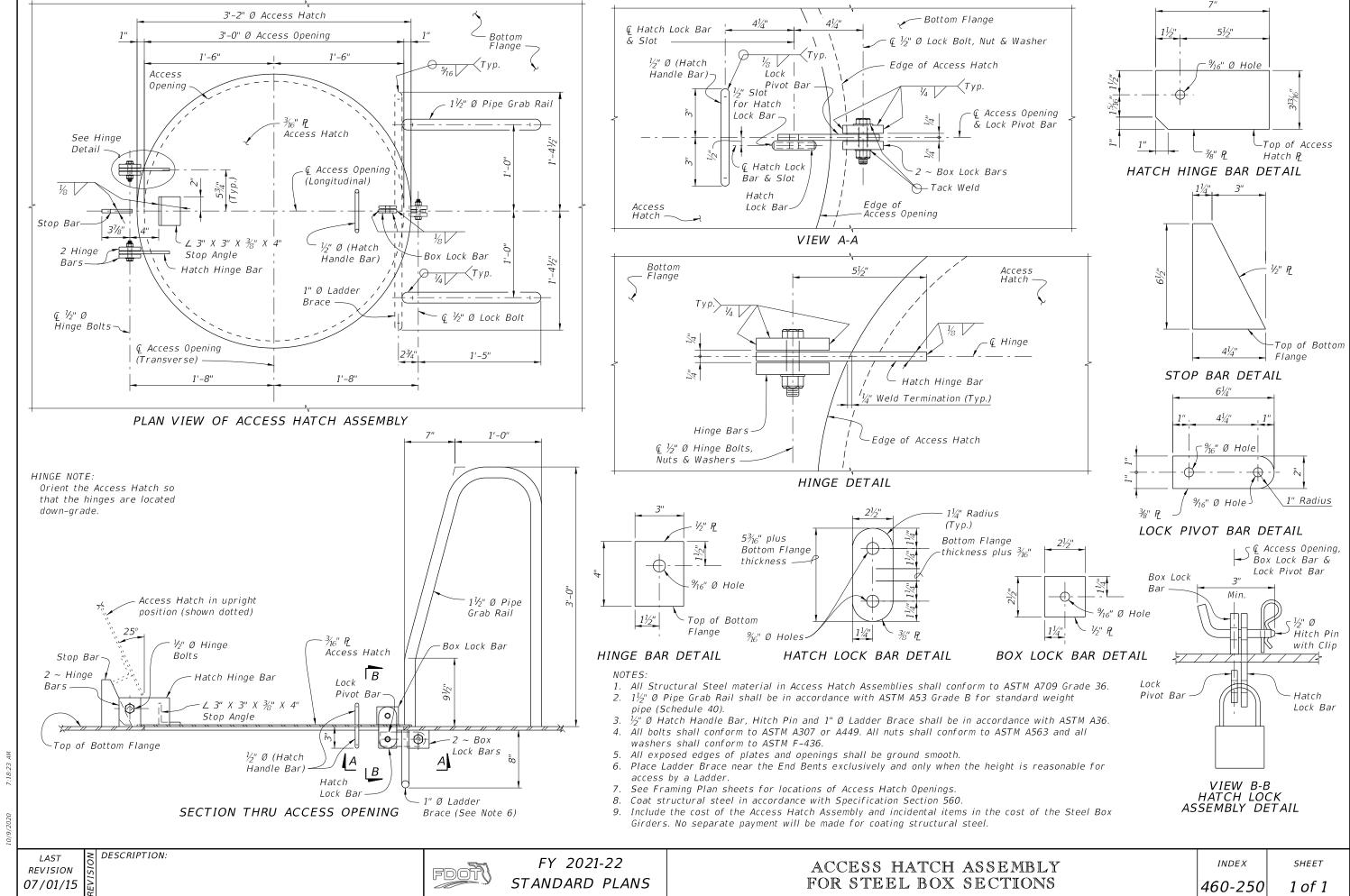
GENERAL NOTES:

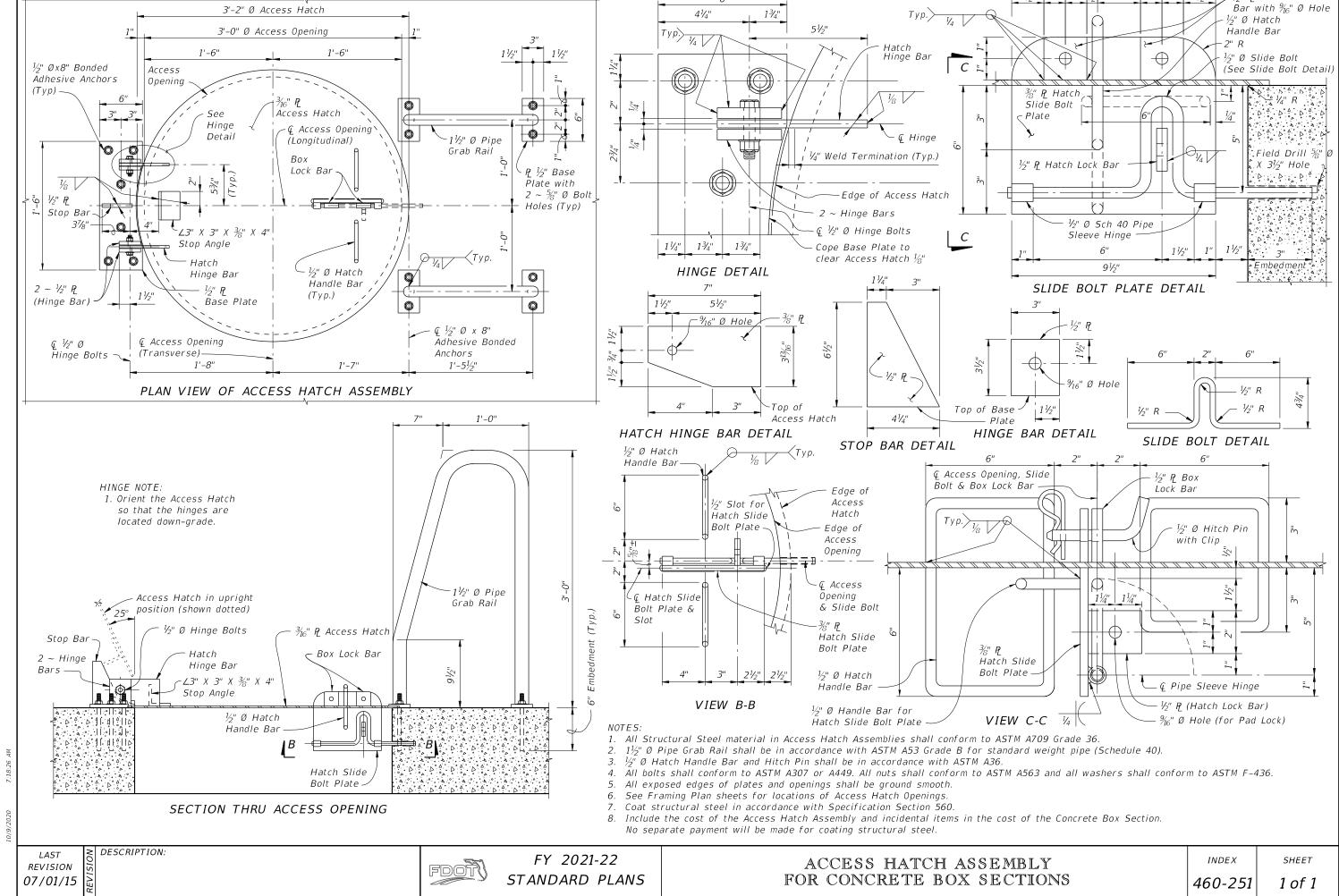
- 1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
- 2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.

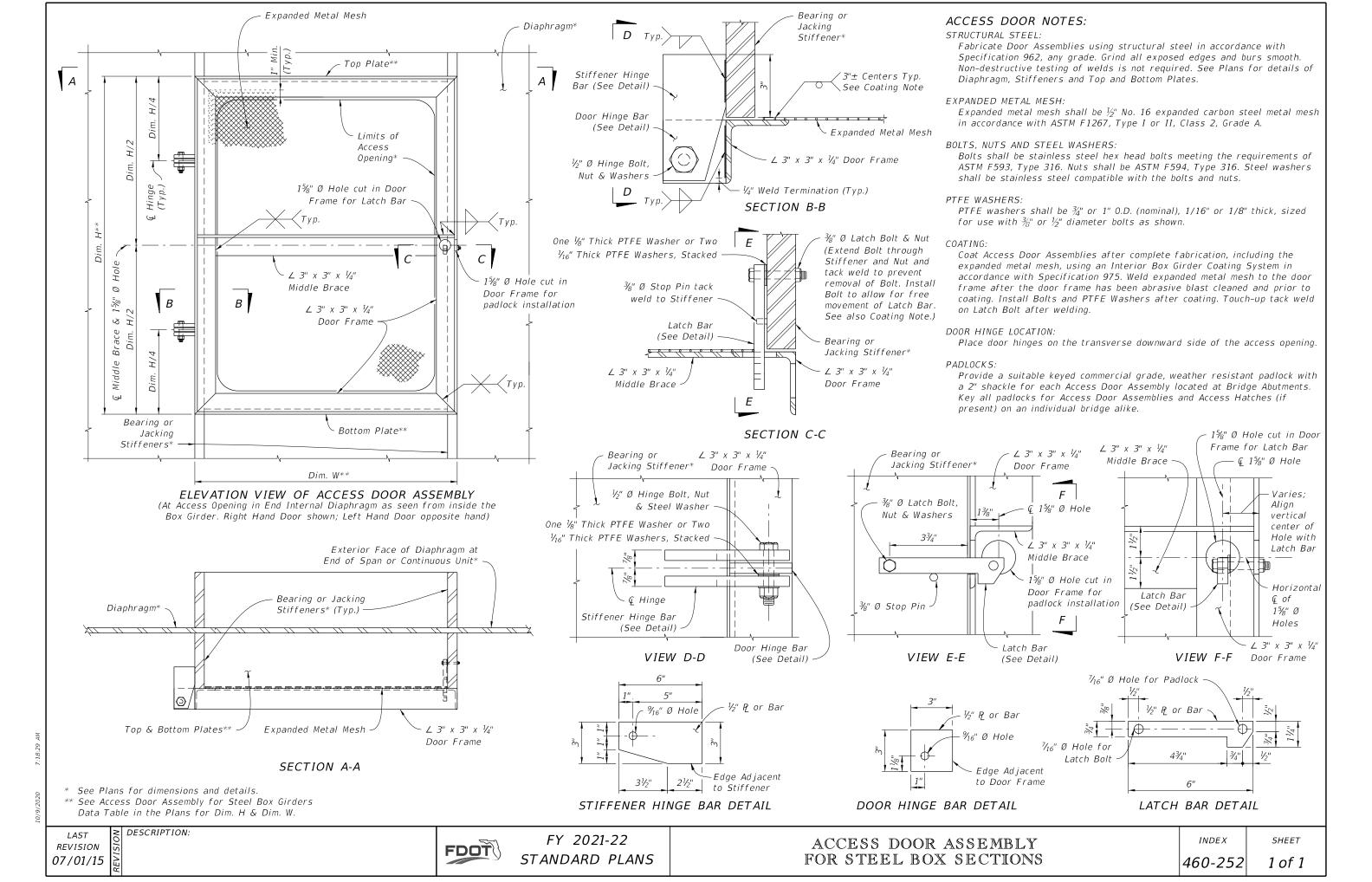
10/9/2020

LAST REVISION 07/01/14









CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be 3'" by 21'," slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM A449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

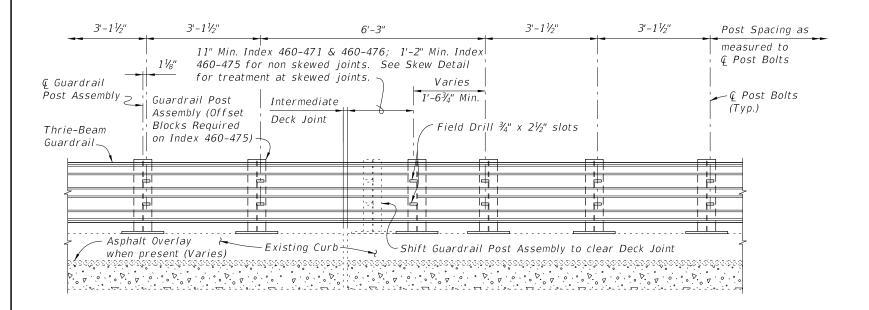
ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for $\frac{1}{4}$ " Ø anchor bolts; 55,000 lbs. for the $\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the $\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

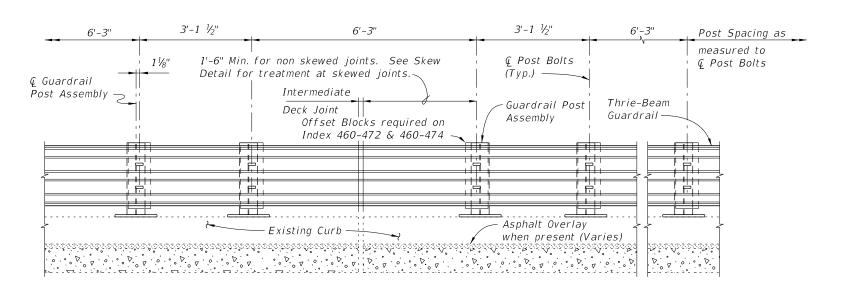
POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1½" spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

- THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.
- BEARING PADS: Provide plain Neoprene pads with a durometer hardness of 60 or 70 and meeting the requirements of Specification Section 932, for ancillary structures.
- ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.
- BARRIER DELINEATORS: Install Barrier Delineators at the top of the guardrail offset blocks in accordance with Specification Section 705. Match the Barrier Delineators color (white or yellow) to the near edgeline.
- PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index 536-001 for details.
- BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

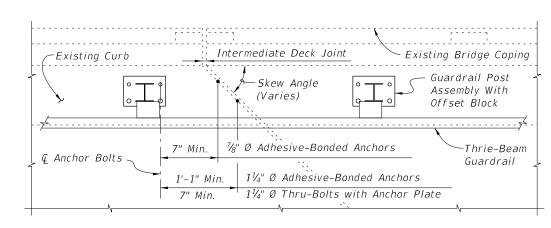
PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.



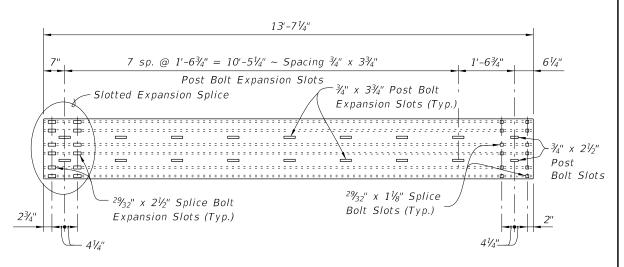
PARTIAL ELEVATION OF INSIDE FACE OF RAILING MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX 460-471, 460-475 & 460-476



PARTIAL ELEVATION OF INSIDE FACE OF RAILING MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX 460-472, 460-473 & 460-474

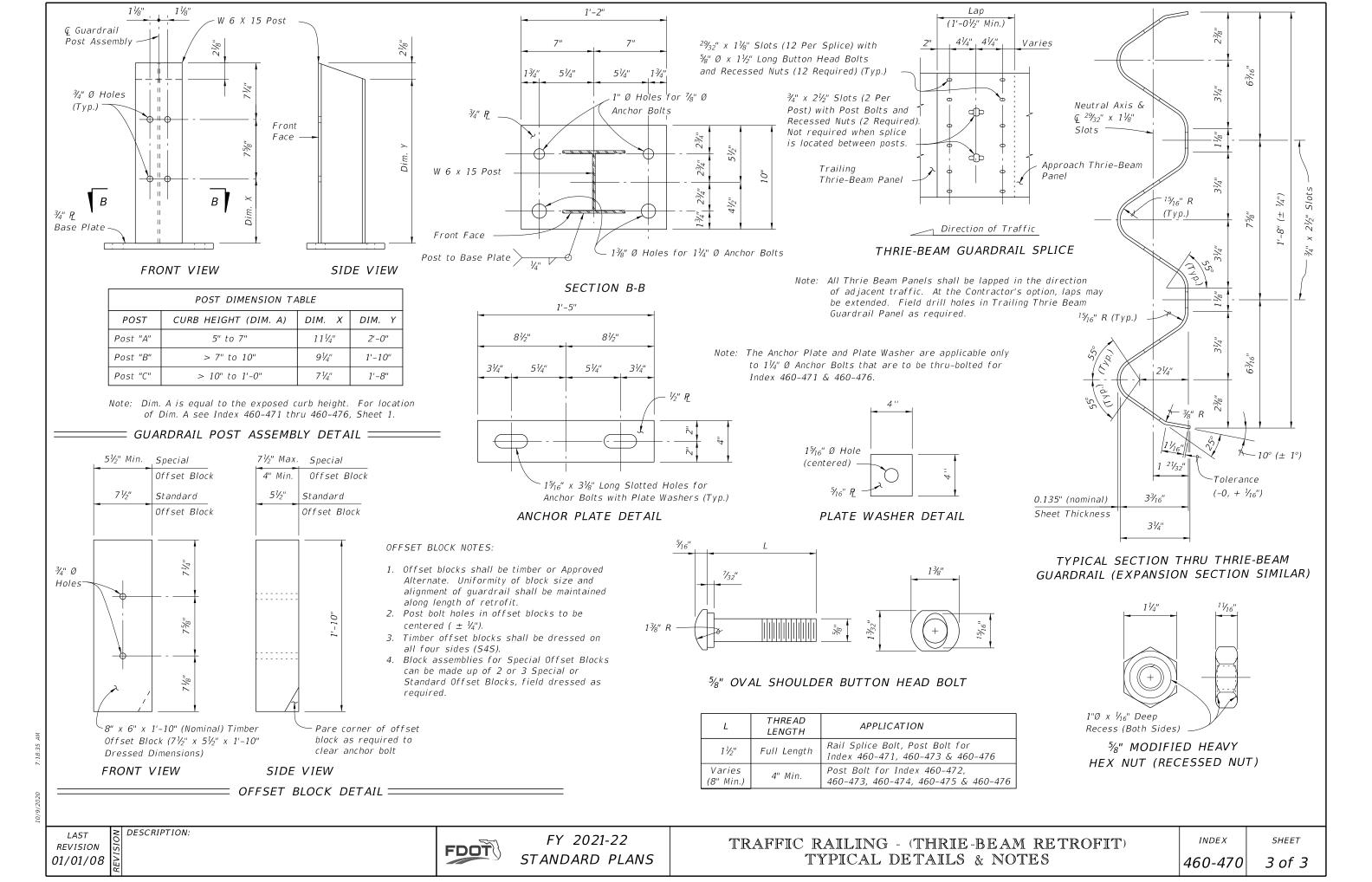


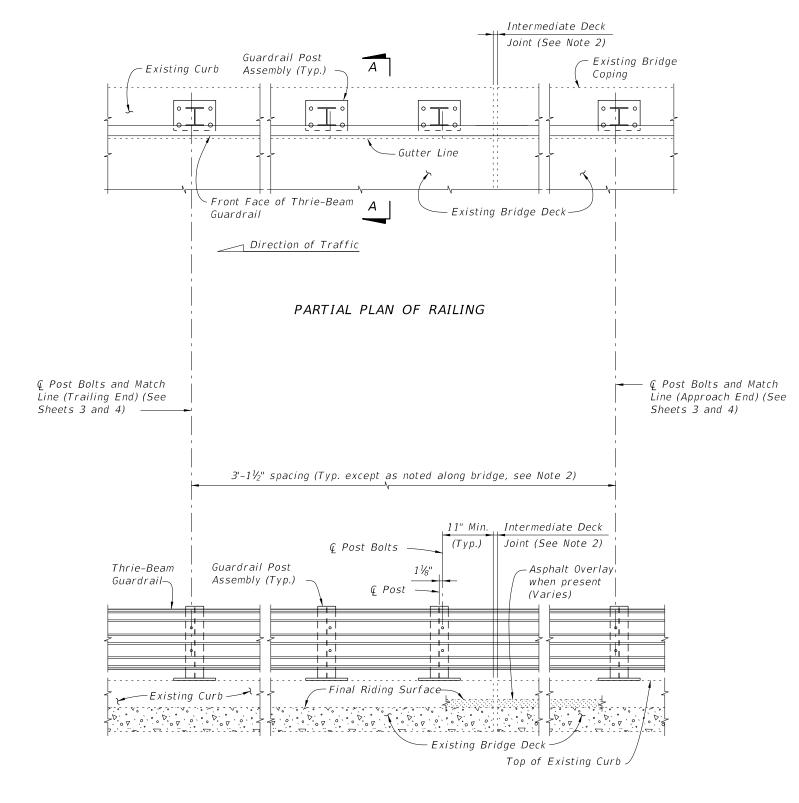
PARTIAL PLAN INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION

DESCRIPTION:





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

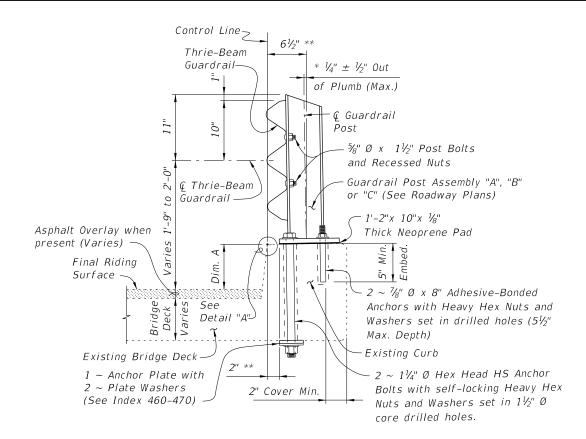
- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

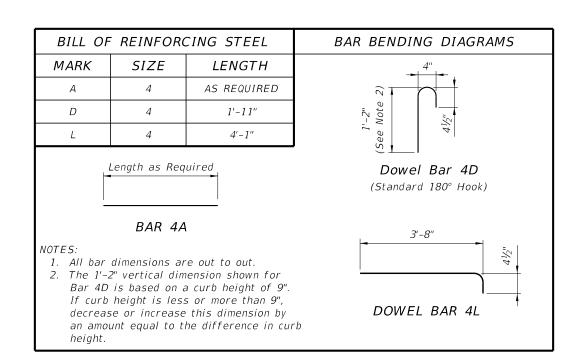
REVISION 01/01/08

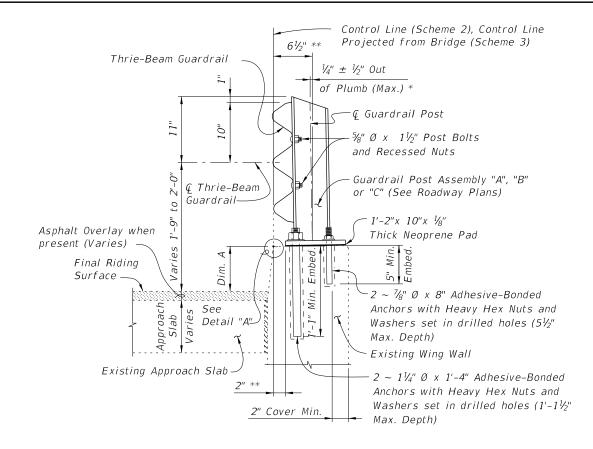
DESCRIPTION:

FY 2021-22 STANDARD PLANS



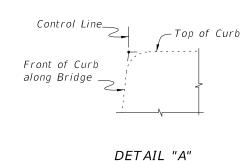
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

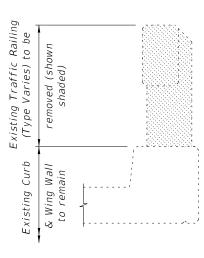




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

REVISION 01/01/08

FDOT

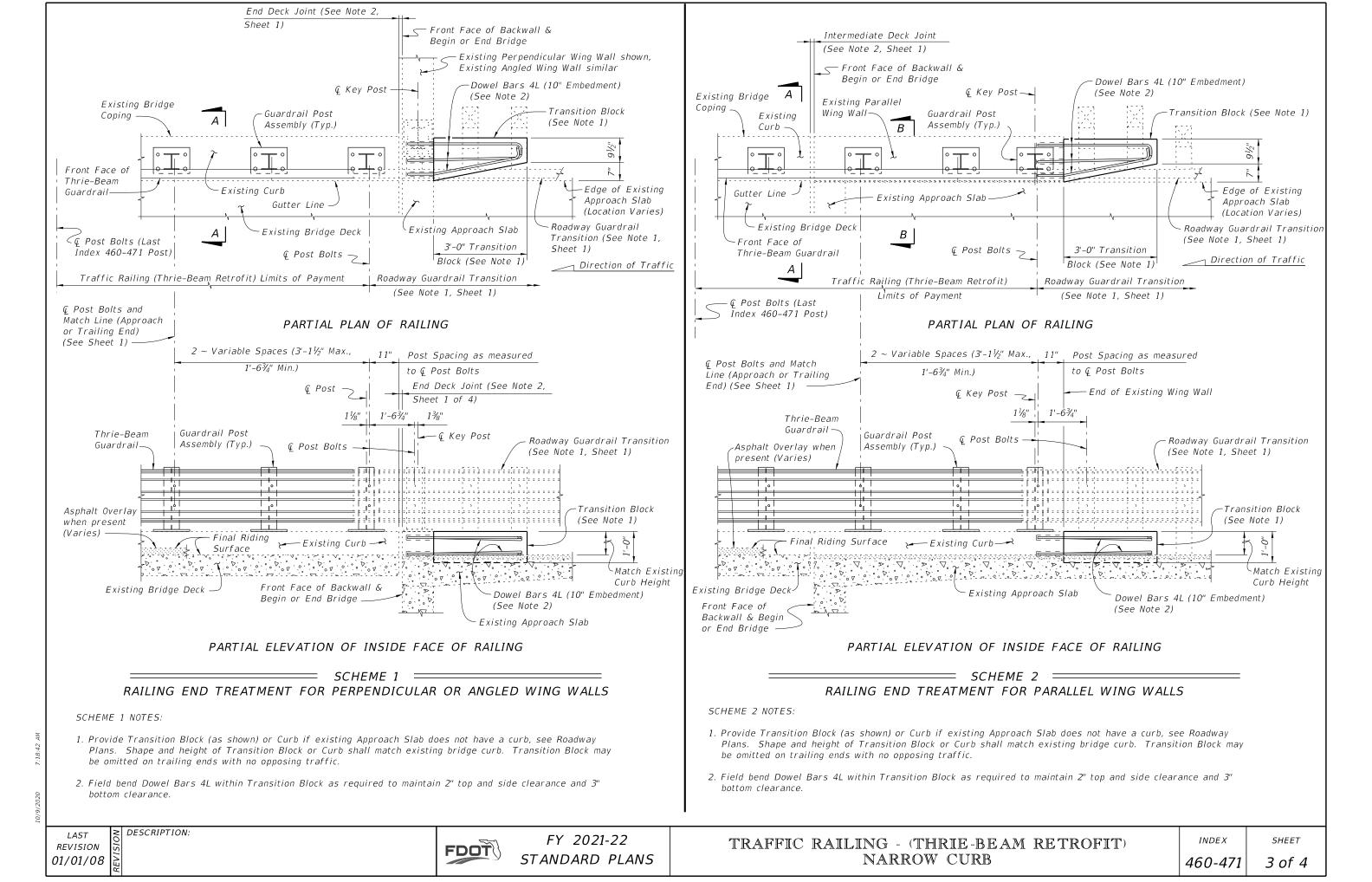
FY 2021-22 STANDARD PLANS

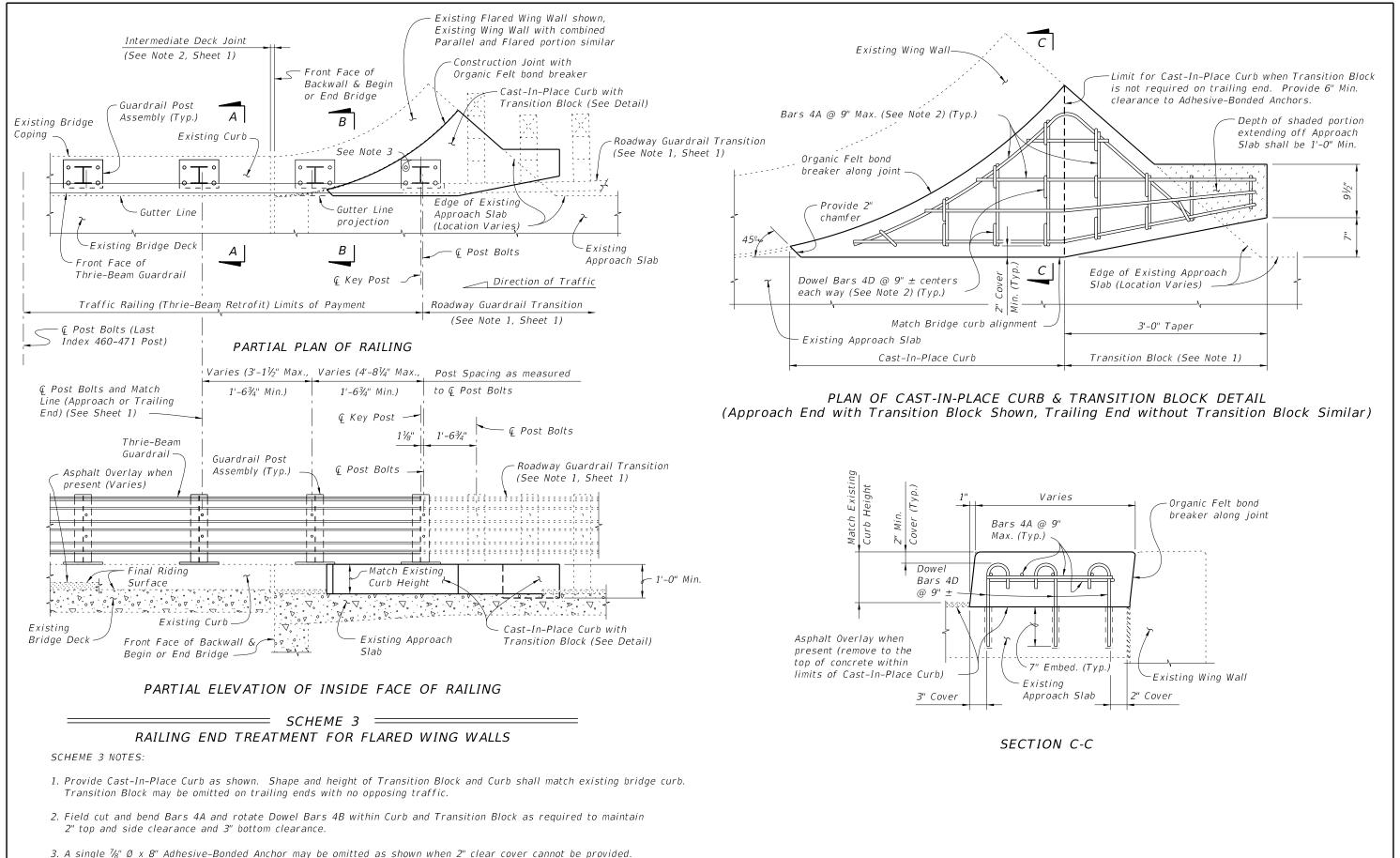
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

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460-471 2 of 4

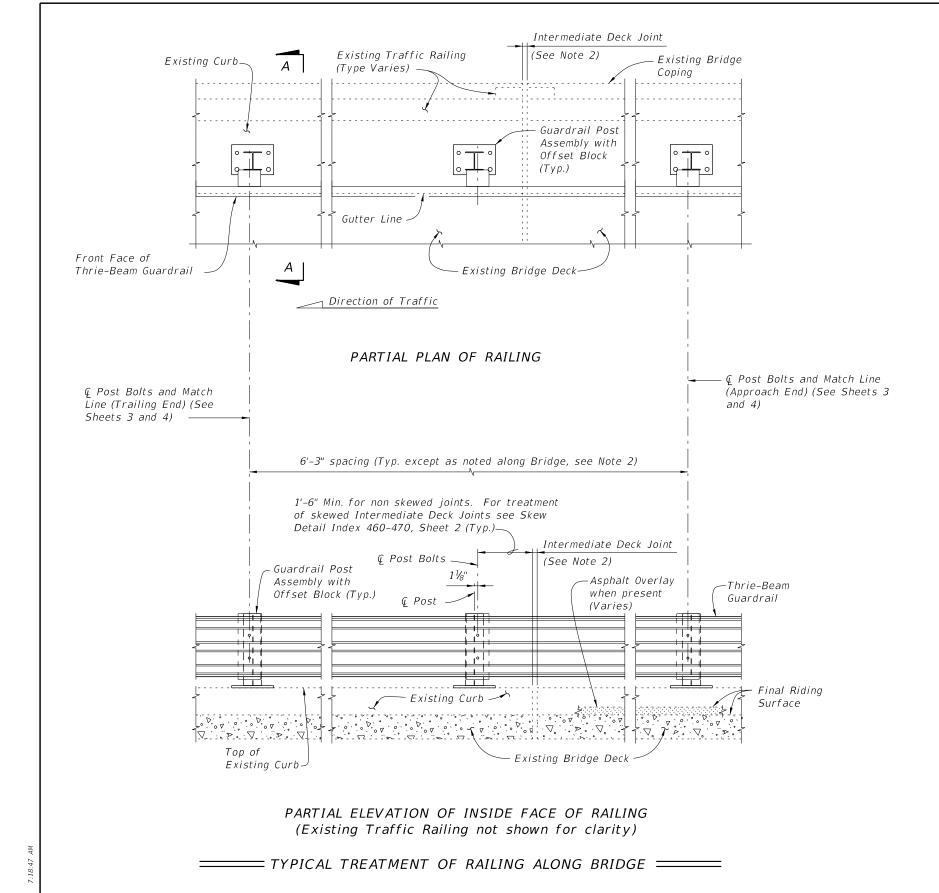




7 0/0/2020

LAST REVISION 11/01/16

DESCRIPTION:



NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

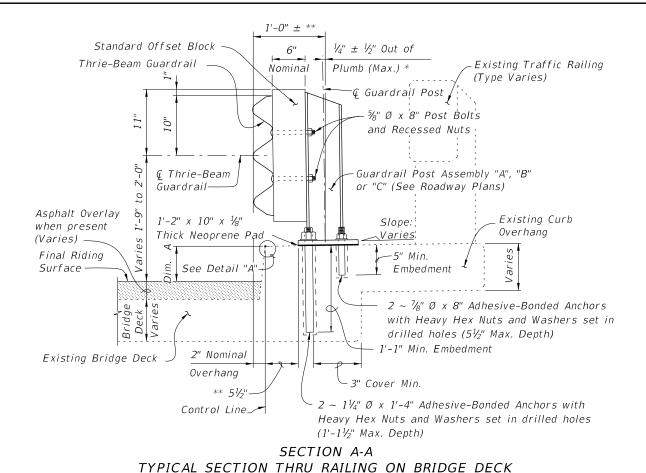
CROSS REFERENCES: For Section A-A see Sheet 2.

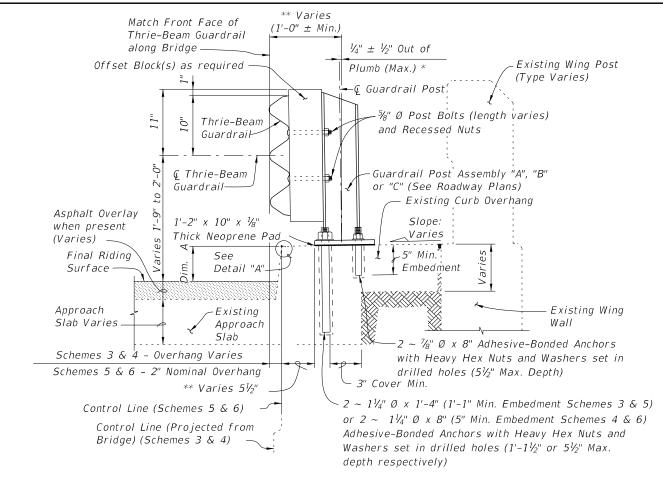
For Traffic Railing Notes and Details see Index 460-470.

REVISION 01/01/08

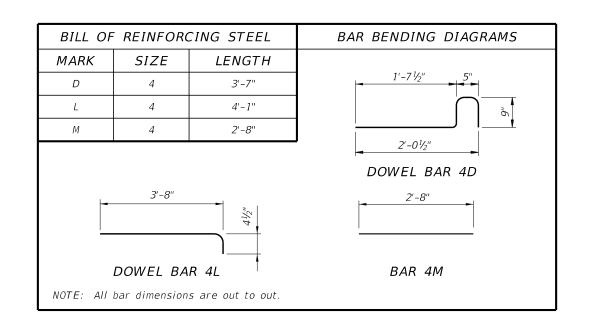
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FY 2021-22 STANDARD PLANS



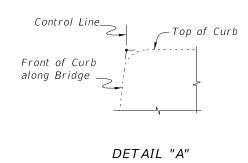


SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)



Shim with washers around Anchors as required to maintain tolerance. Offset may vary ± 1" for Adhesive-Bonded Anchors

to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



Match shape of -Varies (Match existing curb curb height) Asphalt Overlay when present (Varies) 4M Existing 1'-41/2" Approach Slab Dowel Bars 4D (10" Embedment) Edge of Existing (See Note 2, Sheet3, Scheme 2) Approach Slab

VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 07/01/08

DESCRIPTION:

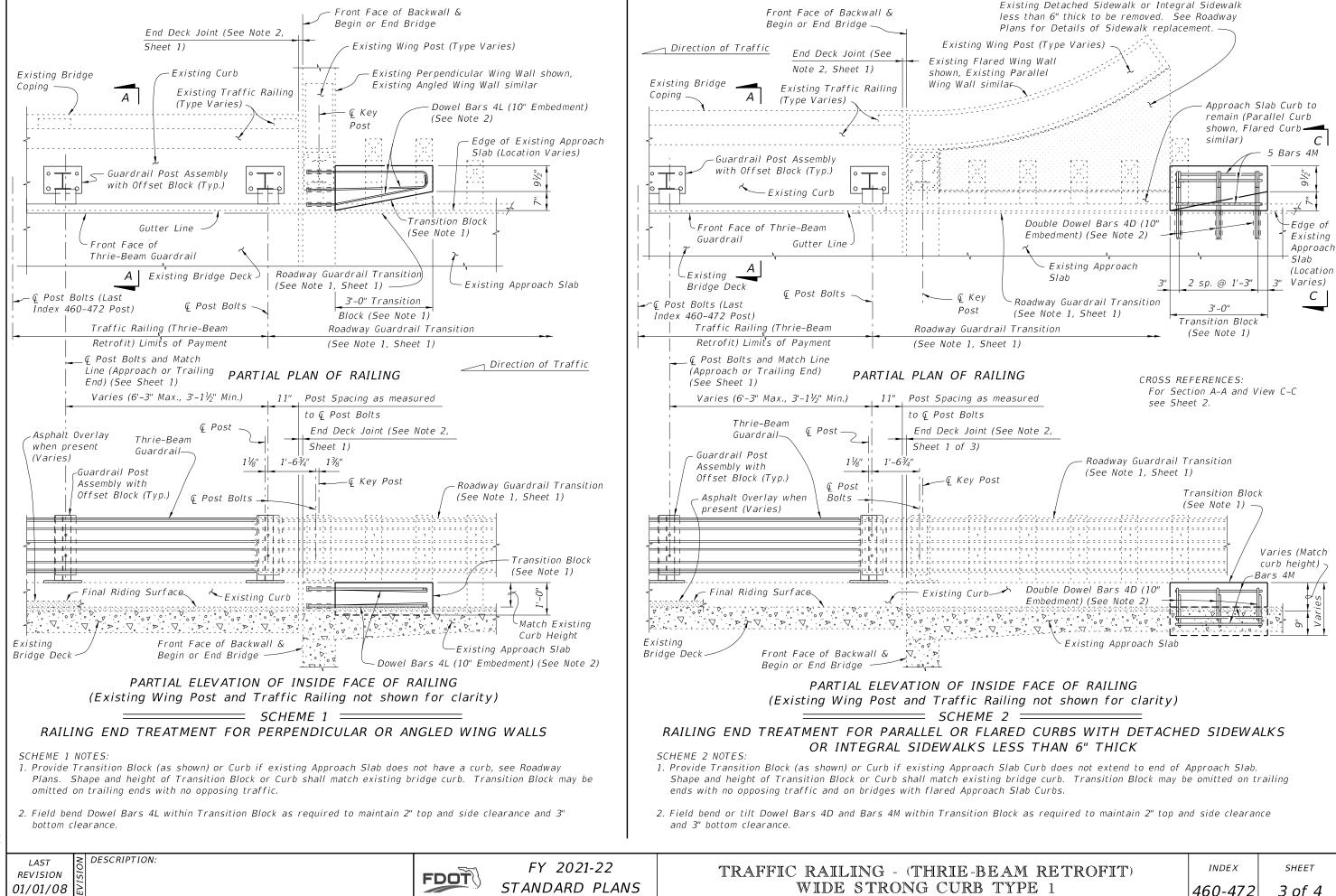
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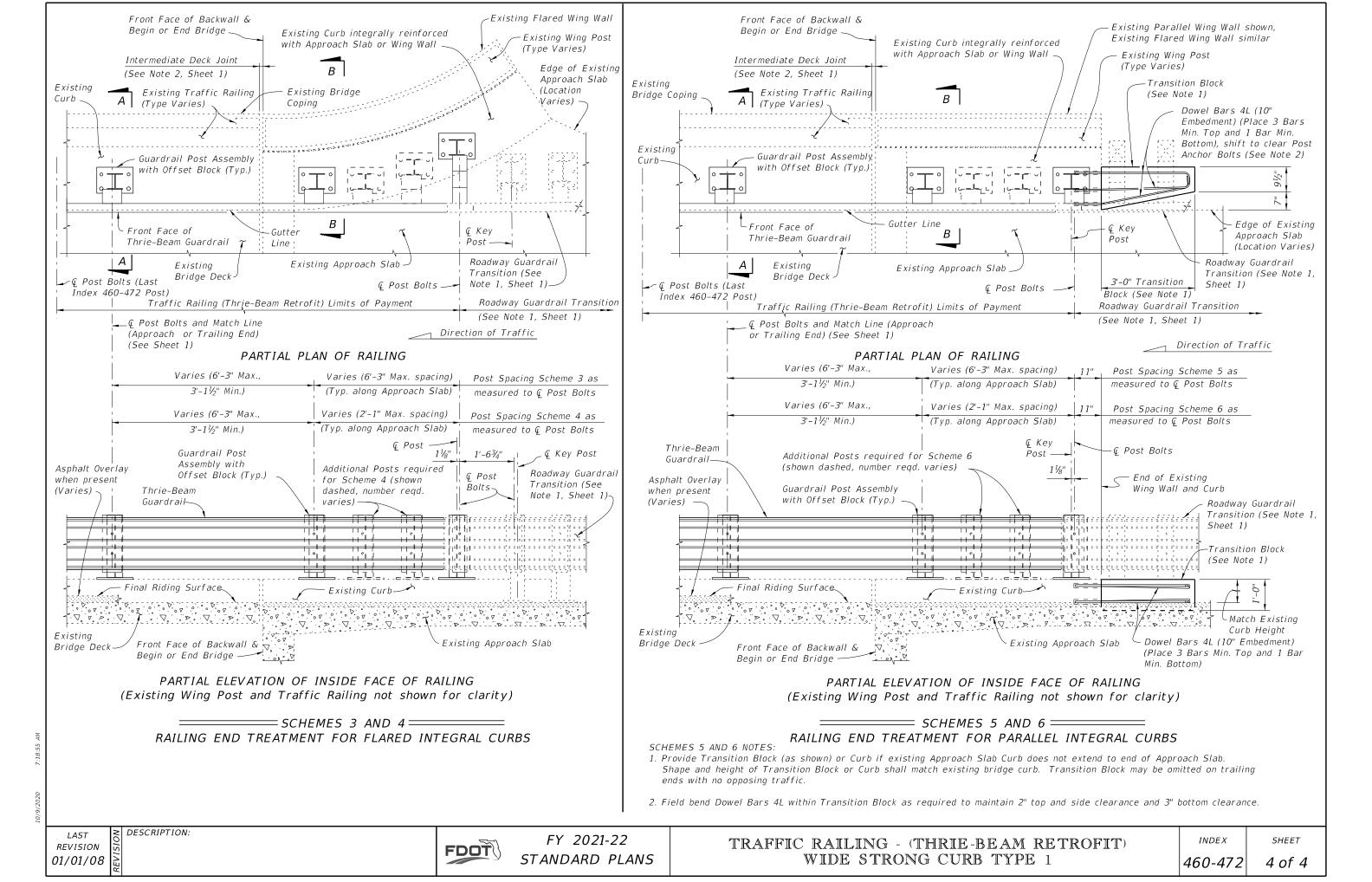
FY 2021-22 STANDARD PLANS

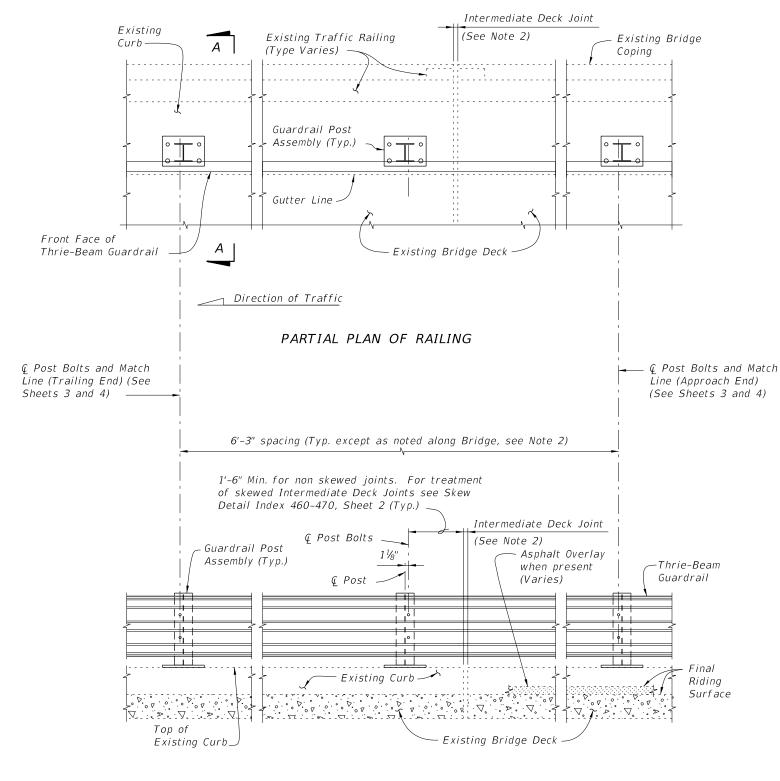
INDEX

SHEET

460-472 2 of 4







PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

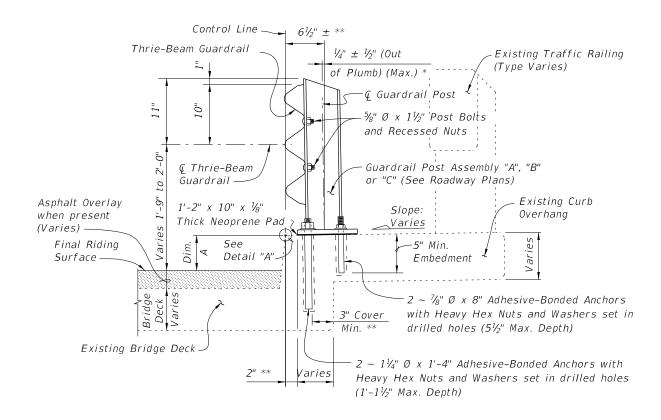
NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

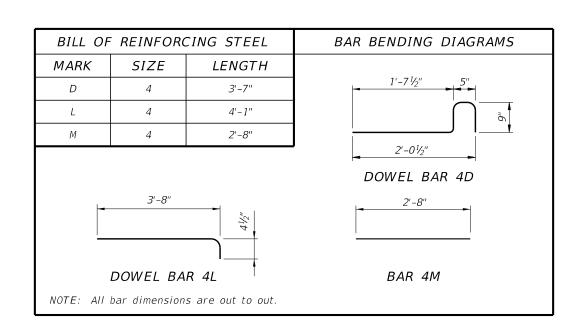
CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

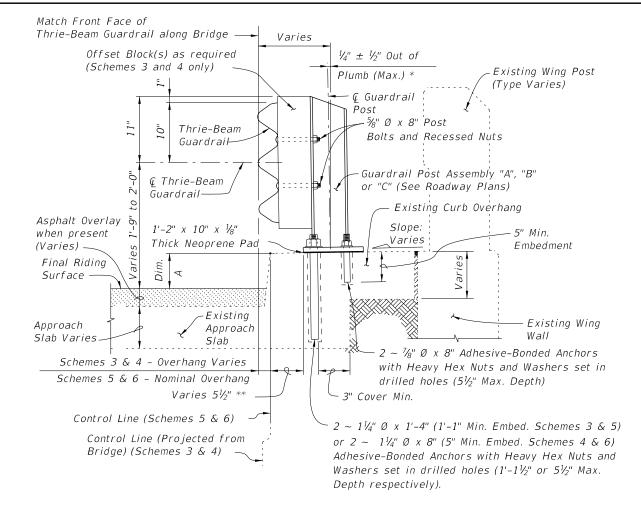
LAST REVISION 01/01/08

DESCRIPTION:



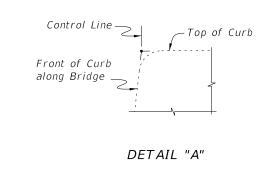
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

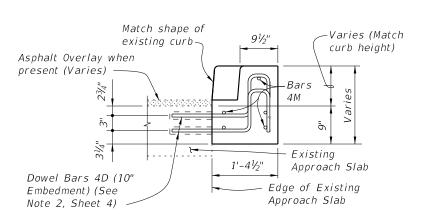




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- * Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- ** Offset may vary \pm 1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index 460-470.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

DESCRIPTION: REVISION

FDOT

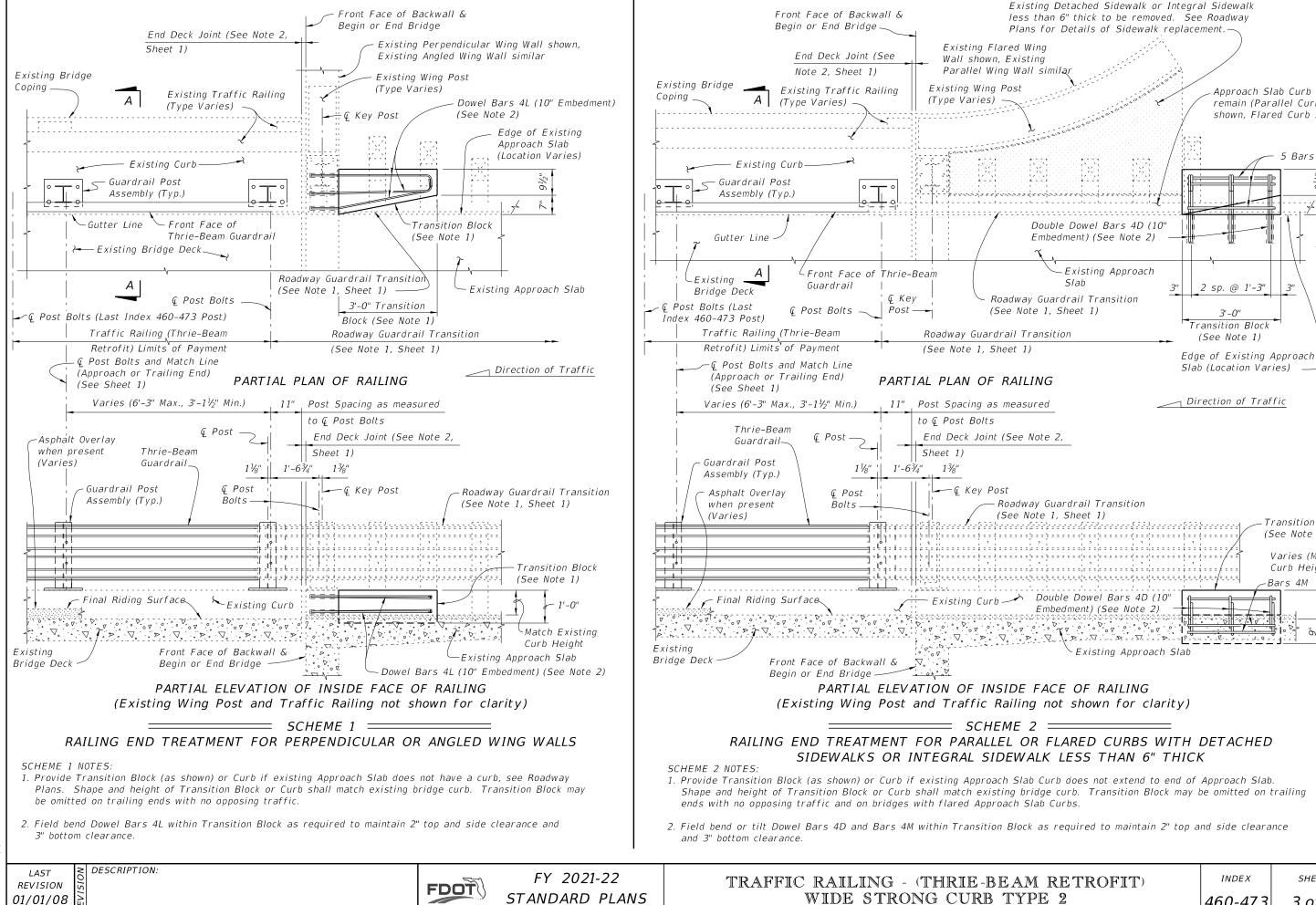
FY 2021-22 STANDARD PLANS

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2

INDEX 460-473

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INDEX 460-473

Approach Slab Curb to

shown, Flared Curb similar,

С

5 Bars 4M

Transition Block

Varies (Match Curb Height) -

(See Note 1)

-Bars 4M

remain (Parallel Curb

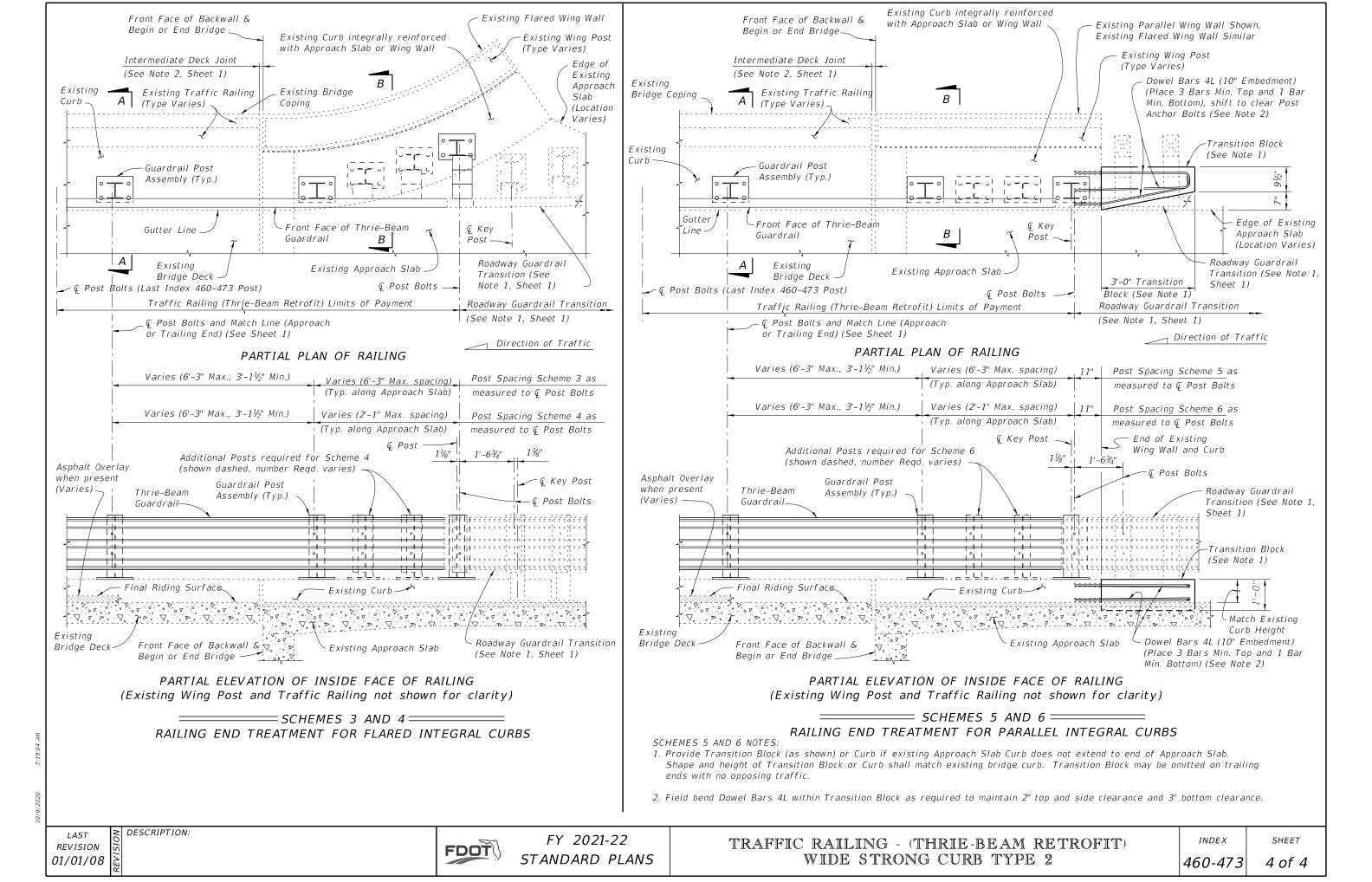
2 sp. @ 1'-3"

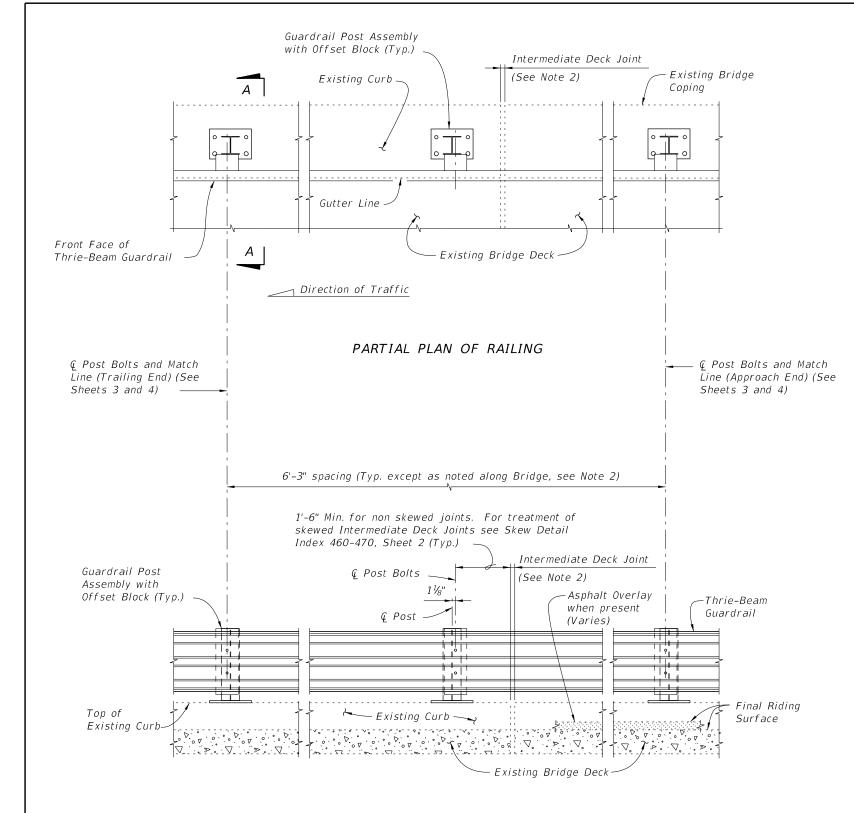
3'-0"

Transition Block

(See Note 1)

SHEET 3 of 4





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

===== TYPICAL TREATMENT OF RAILING ALONG BRIDGE =======

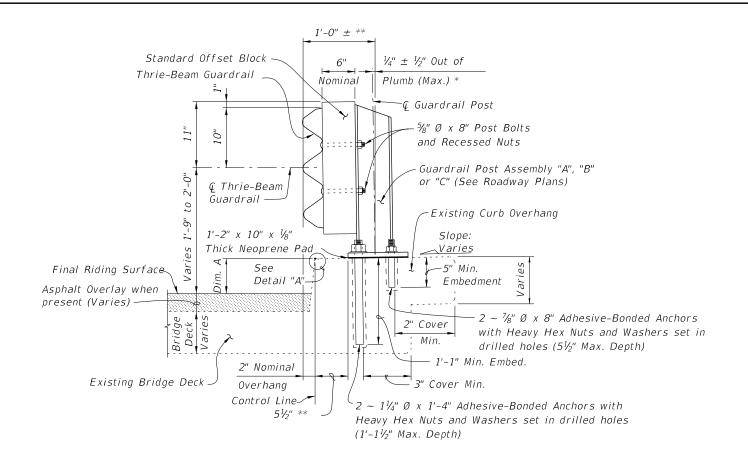
NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

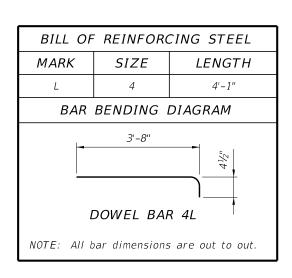
CROSS REFERENCES: For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

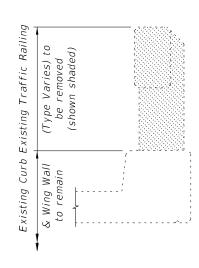
REVISION 01/01/08

DESCRIPTION:

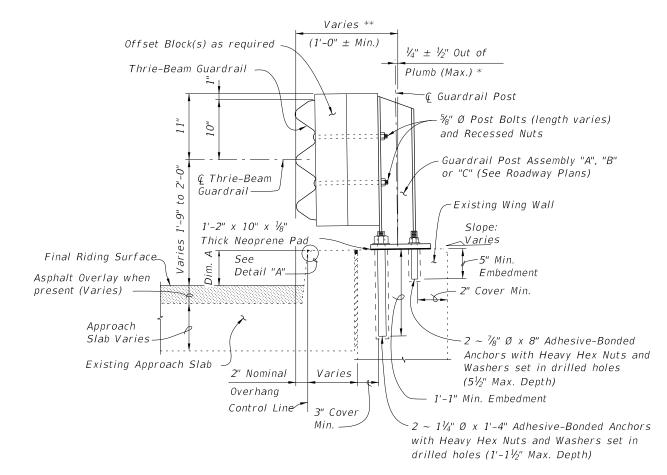


SECTION A-A
TYPICAL SECTION THRU RAILING ON BRIDGE DECK





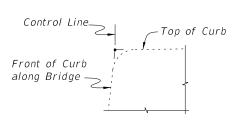
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION B-B (SCHEME 2)
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

*Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

***Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



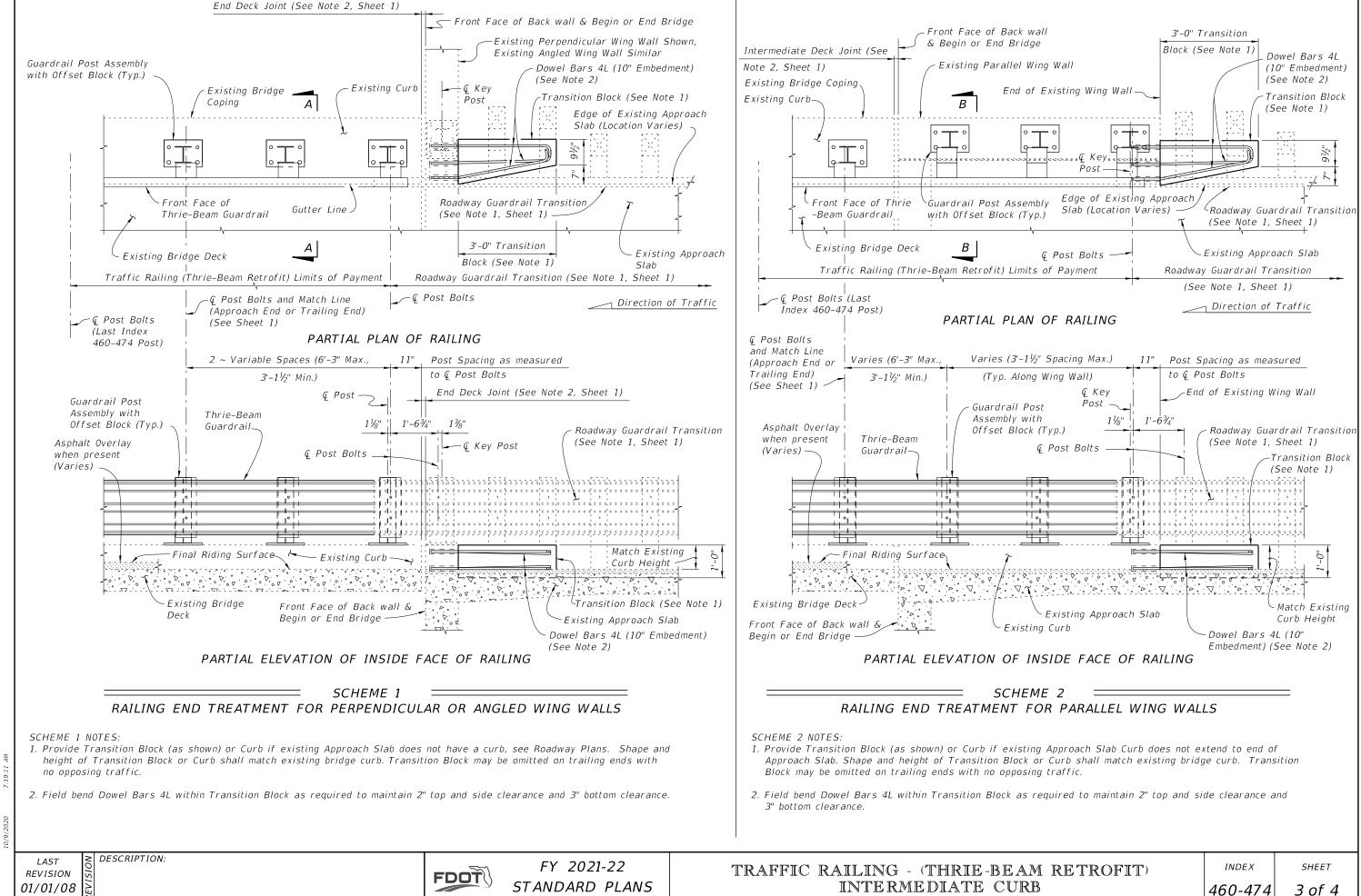
DETAIL "A"

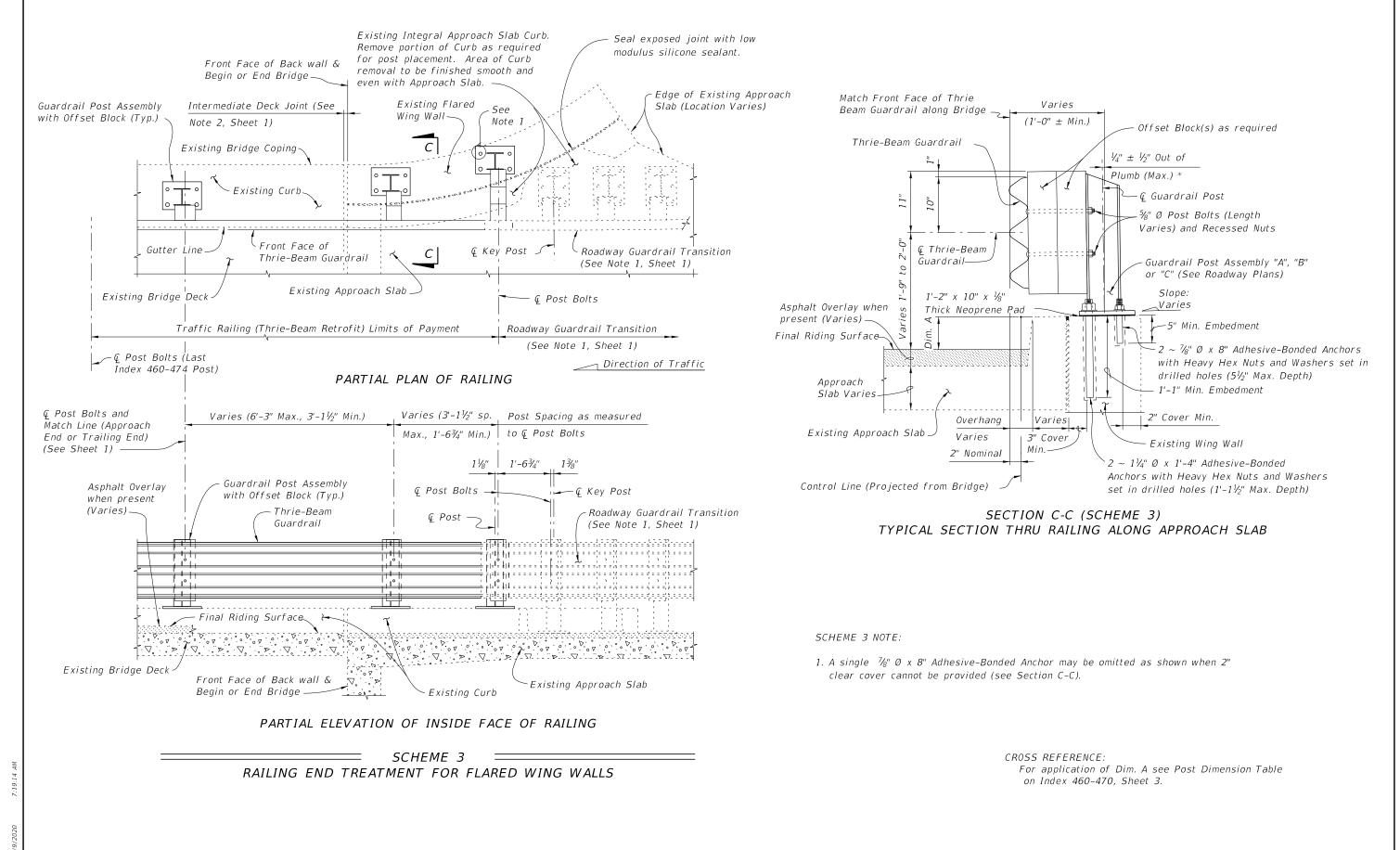
CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3. For location of Section B-B see Sheet 3 For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

DESCRIPTION:

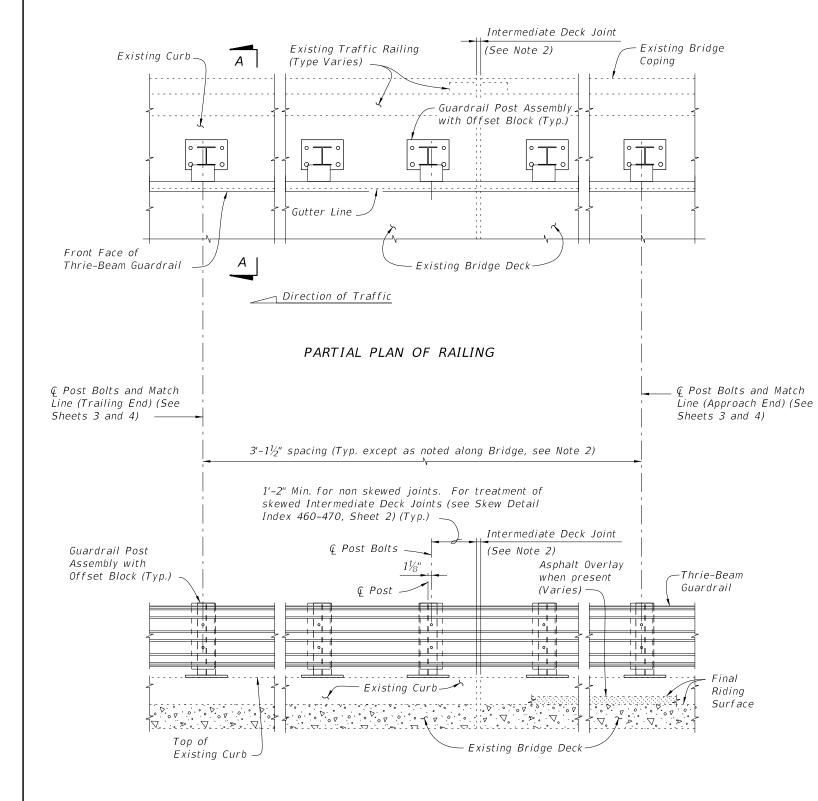
SHEET





LAST REVISION 07/01/09

DESCRIPTION:



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

= TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

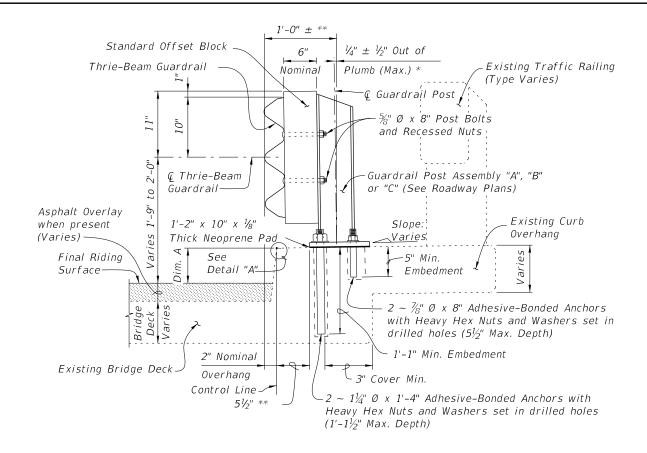
- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

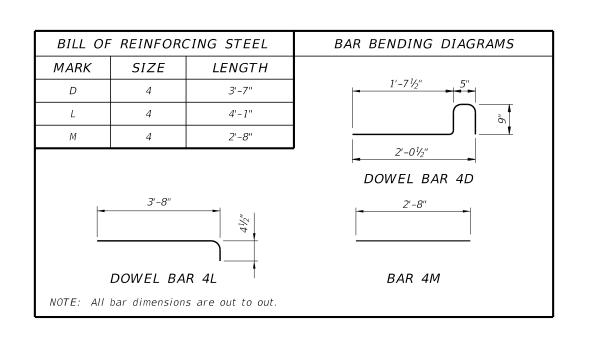
REVISION 01/01/08

DESCRIPTION:





SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

when present

Surface

Approach

Slab Varies

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

Control Line (Schemes 5 & 6) -

Control Line (Projected from

Bridge) (Schemes 3 & 4) -

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required

Thrie-Beam

Guardrail-

♀ Thrie-Beam

1'-2" x 10" x 1/8"

Thick Neoprene Pad See

Detail "A'

-Existing

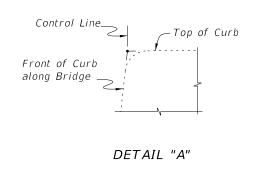
Approach

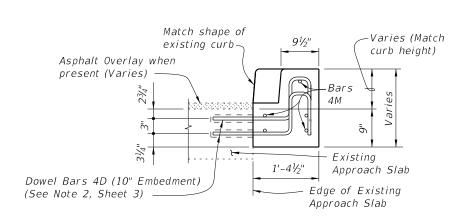
Slab

Varies 51/3" **

Guardrail-

Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

Varies **

 $(1'-0'' \pm Min.)$

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

Plumb (Max.) *

-Ç Guardrail Post:

and Recessed Nuts

Slope:

Embedment

<u>___Varie</u>s

_5" Min.

└ 3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

%" Ø Post Bolts (length varies)

-Guardrail Post Assembly "A", "B"

Existing Curb Overhang

 $\sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5)

or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}'')$ or $5\frac{1}{2}''$ Max.

drilled holes (5½" Max. Depth)

or "C" (See Roadway Plans)

Existing Wing Post

Existing Wing

(Type Varies)

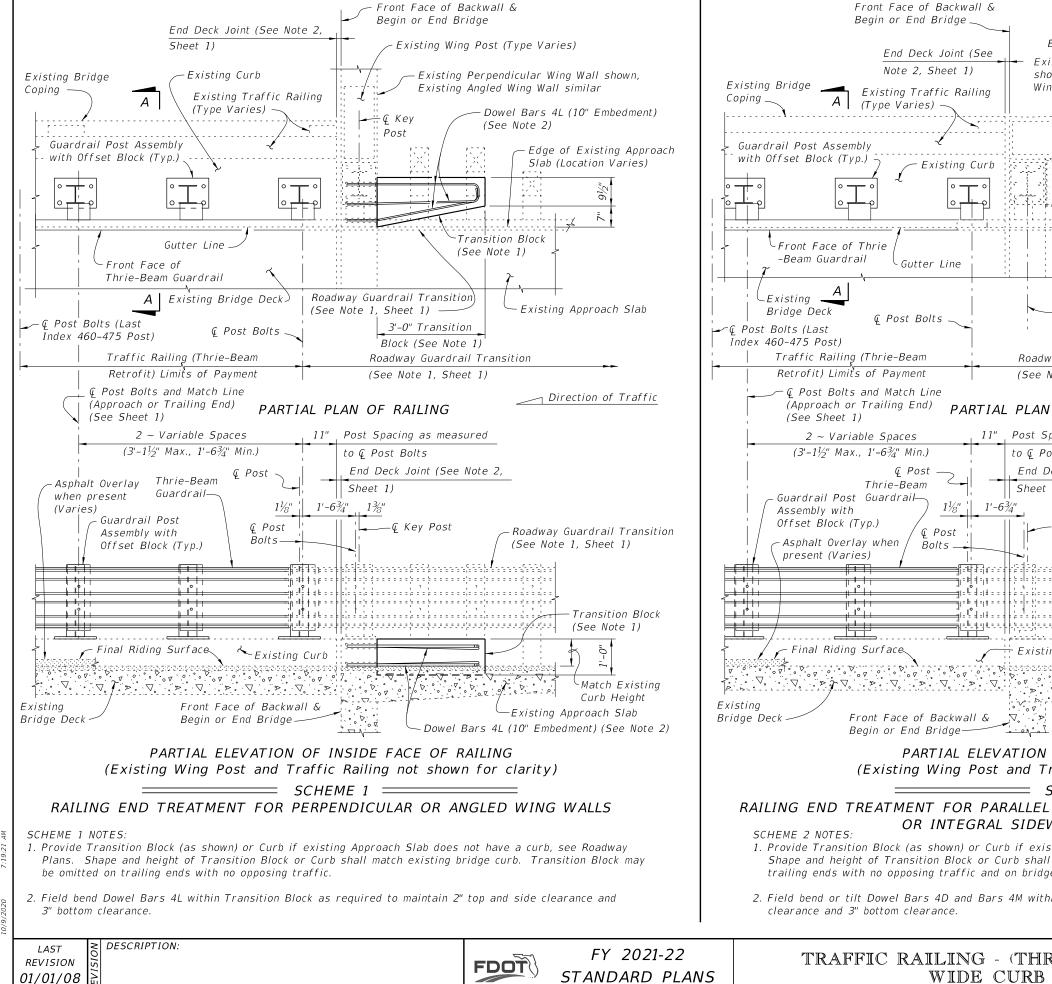
For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

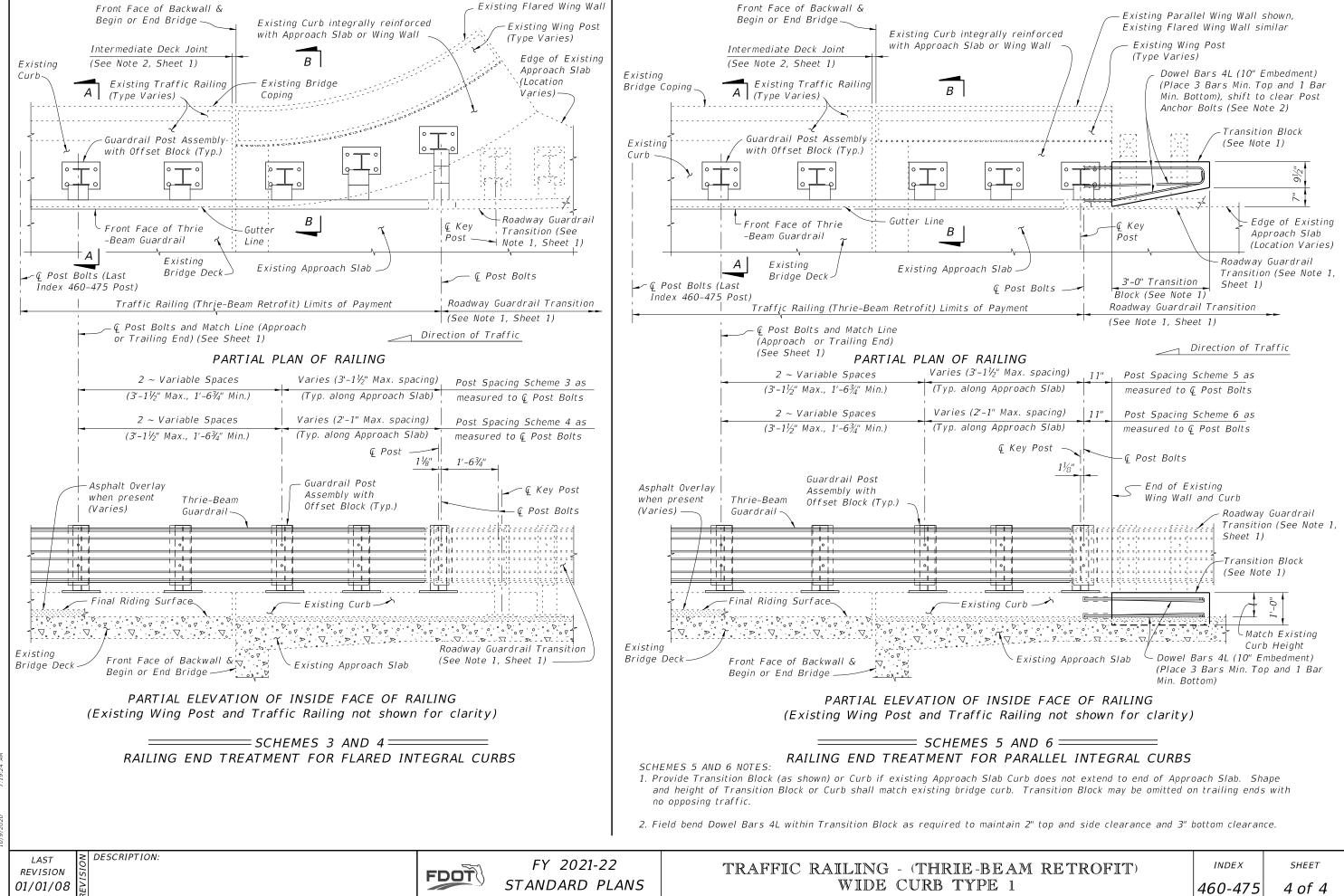
DESCRIPTION: REVISION 01/01/08

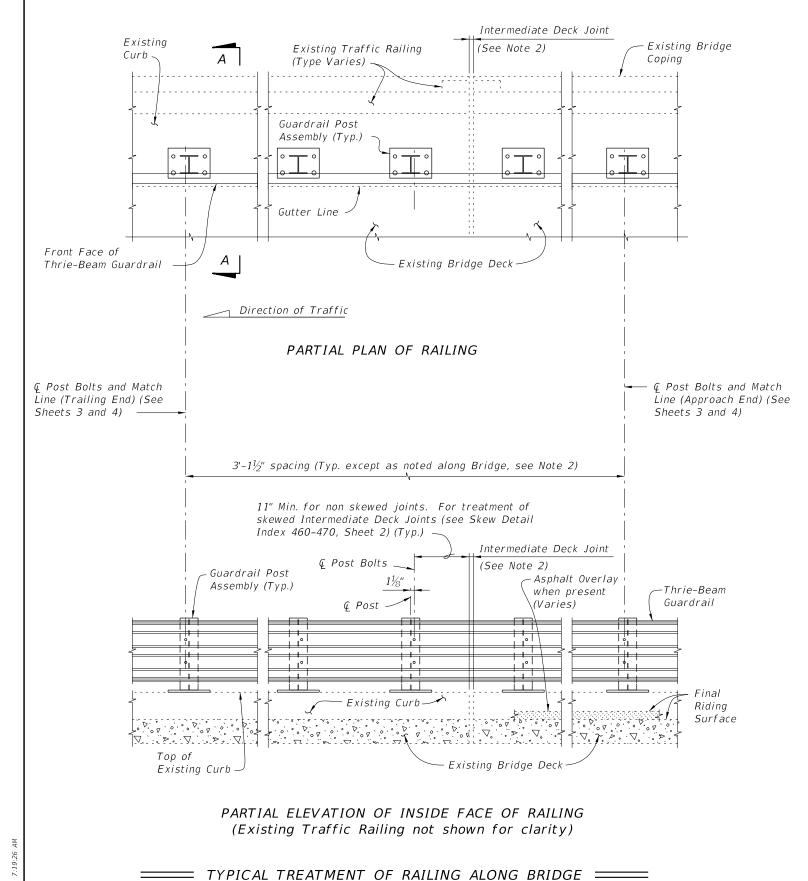


less than 6" thick to be removed. See Roadway Plans for Details of Sidewalk replacement. Existing Wing Post (Type Varies) Existing Flared Wing Wall shown, Existing Parallel Wing Wall similar Approach Slab Curb to remain (Parallel Curb shown, Flared Curb similar) С 5 Bars 4M 9½" Double Dowel Bars 4D (10" Edge of Embedment) (See Note 2) Existing Approach Slab Existing Approach (Location Slab 2 sp. @ 1'-3" *Varies)* @ Kev С Post Roadway Guardrail Transition (See Note 1, Sheet 1) Transition Block Roadway Guardrail Transition (See Note 1) (See Note 1, Sheet 1) → Direction of Traffic PARTIAL PLAN OF RAILING 11" Post Spacing as measured to & Post Bolts End Deck Joint (See Note 2, Sheet 1) _@ Key Post Roadway Guardrail Transition (See Note 1, Sheet 1) Transition Block (See Note 1) Varies (Match Curb Height)--Bars 4M Existing Curb --- Double Dowel Bars 4D (10" Embedment) (See Note 2) Existing Approach Slab PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post and Traffic Railing not shown for clarity) ______ SCHEME 2 ____ RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALKS LESS THAN 6" THICK 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs. 2. Field bend or tilt Dowel Bars 4D and Bars 4M within Transition Block as required to maintain 2" top and side SHEET *INDEX*

Existing Detached Sidewalk or Integral Sidewalk

STANDARD PLANS





NOTES:

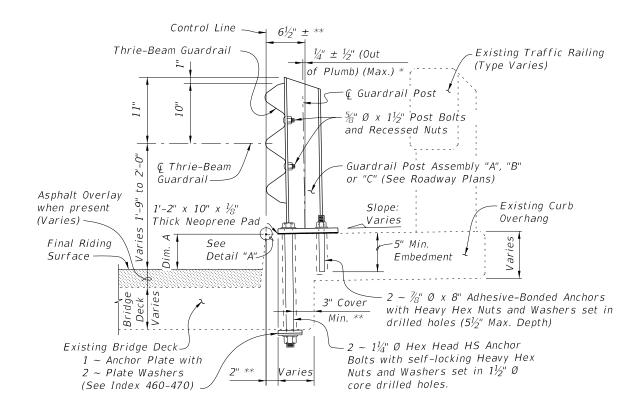
- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

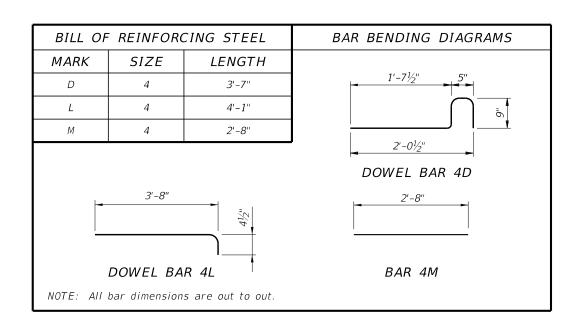
LAST REVISION 01/01/08

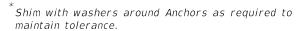
DESCRIPTION:

FI



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

Surface -

Approach

Slab Varies-

when present

(Varies) -

Thrie-Beam Guardrail along Bridge 🗲

Offset Block(s) as required

Thrie-Beam

Guardrail-

 ← Thrie-Beam

1'-2" x 10" x 1/3"

Thick Neoprene Pad

-Existing

Approach

Slab

Varies 51/2" **

Guardrail

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - Nominal Overhang

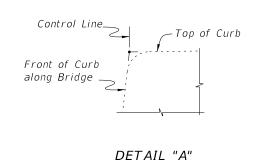
Control Line (Schemes 5 & 6)

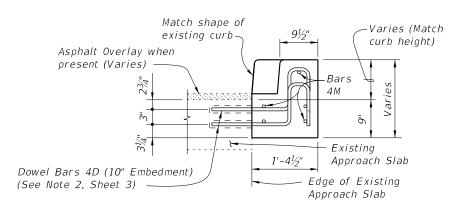
Control Line (Projected from

Bridge) (Schemes 3 & 4)

(Schemes 3 and 4 only)

Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

• © Guardrail

%" Ø x 8" Post

Slope:

Depth respectively).

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Bolts and Recessed Nuts

or "C" (See Roadway Plans)

Guardrail Post Assembly "A", "B"

Varies¦ Embedment

Existing Curb Overhang

__ 5" Min.

drilled holes (5½" Max. Depth)

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5) or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}'')$ or $5\frac{1}{2}''$ Max.

Plumb (Max.) *

Existing Wing Post

Existing Wing

Wall

%" Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

(Type Varies)

For location of Section C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 07/01/08

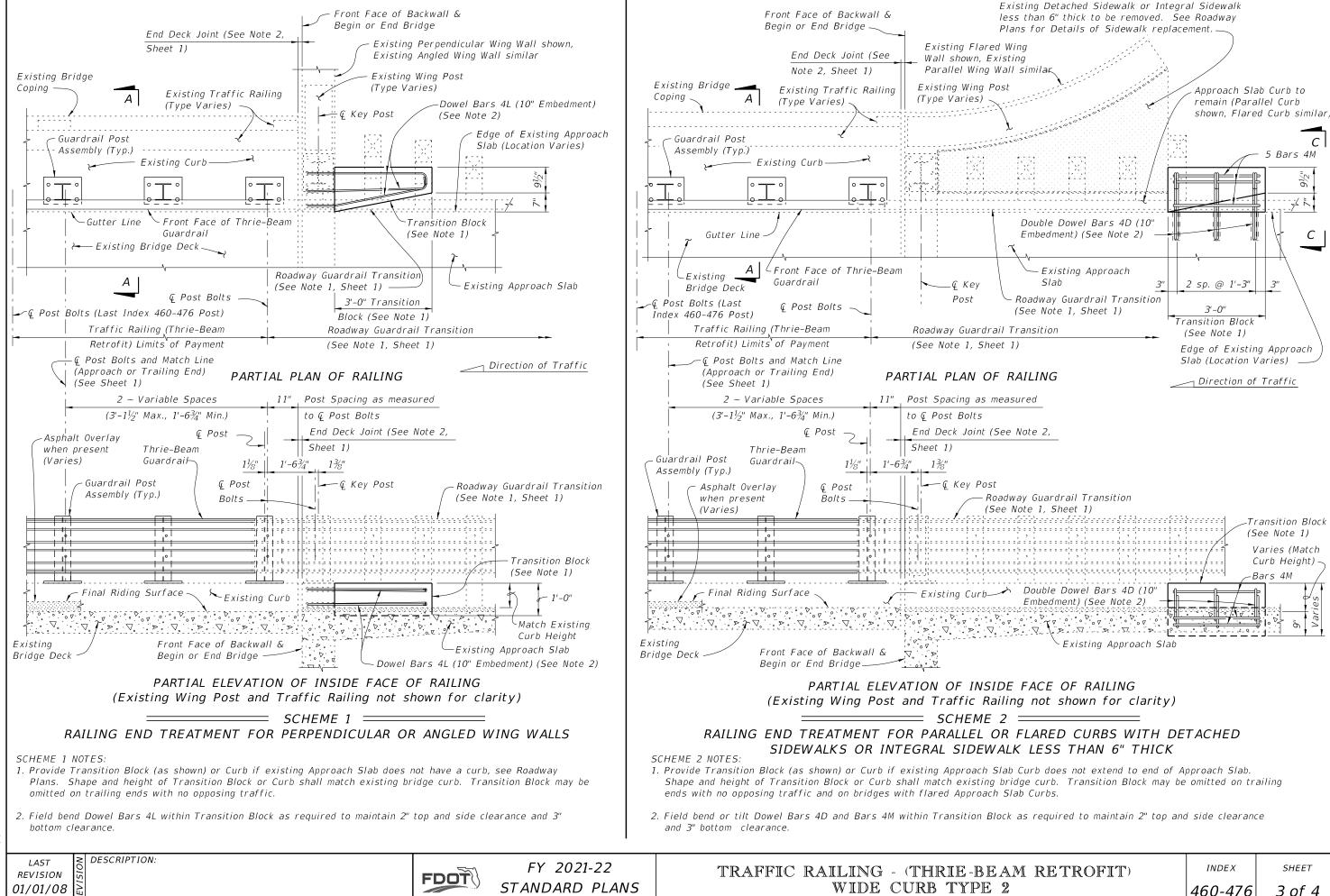
DESCRIPTION:

FDOT

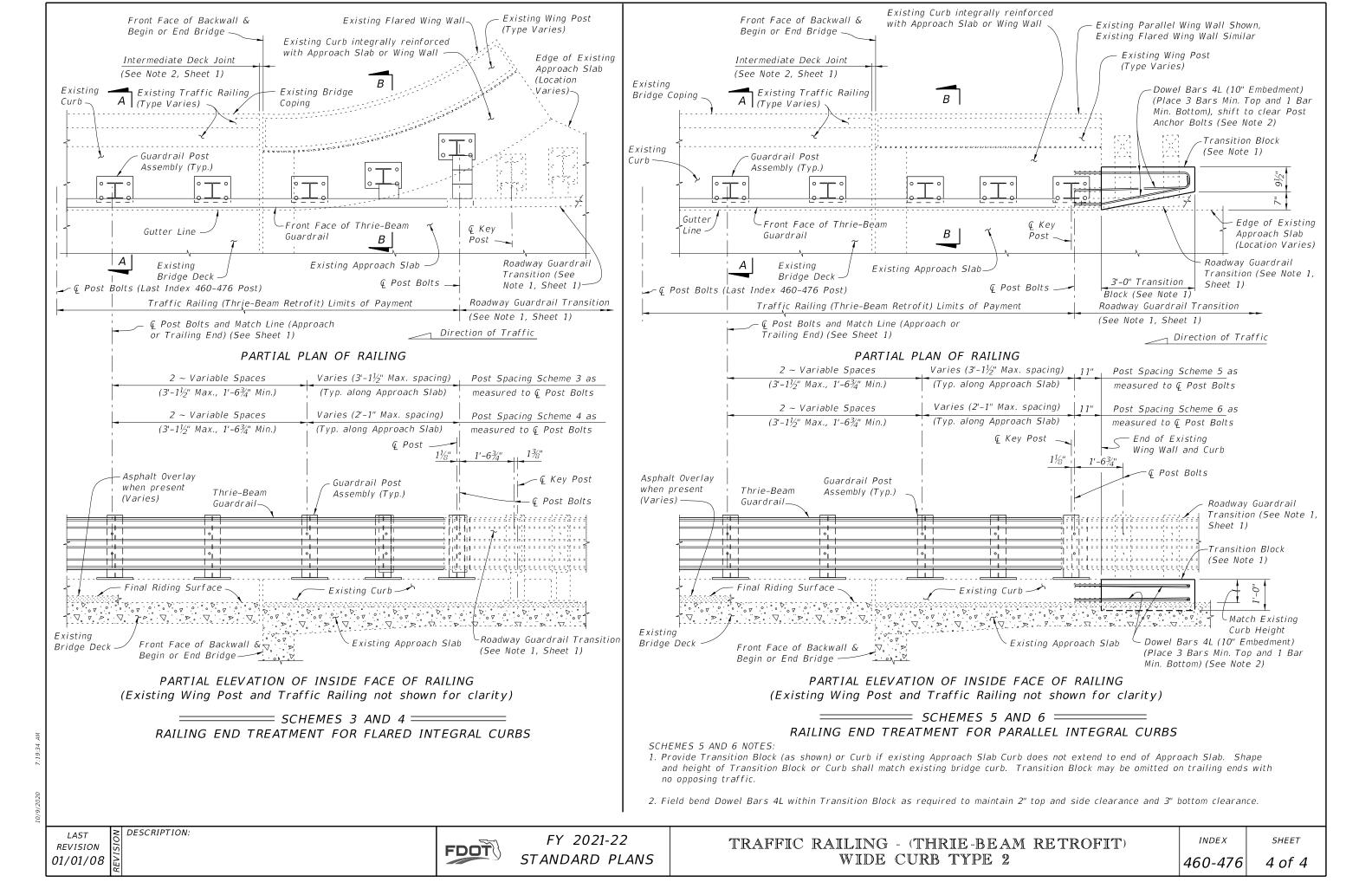
FY 2021-22 STANDARD PLANS TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 2

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SHEET



10/8/01



CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be $\frac{3}{4}$ " by $2\frac{1}{2}$ " slotted holes.

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

COATINGS: All Nuts, Bolts, Anchors, and Washers shall be hot-dip galvanized in accordance with the Specifications.

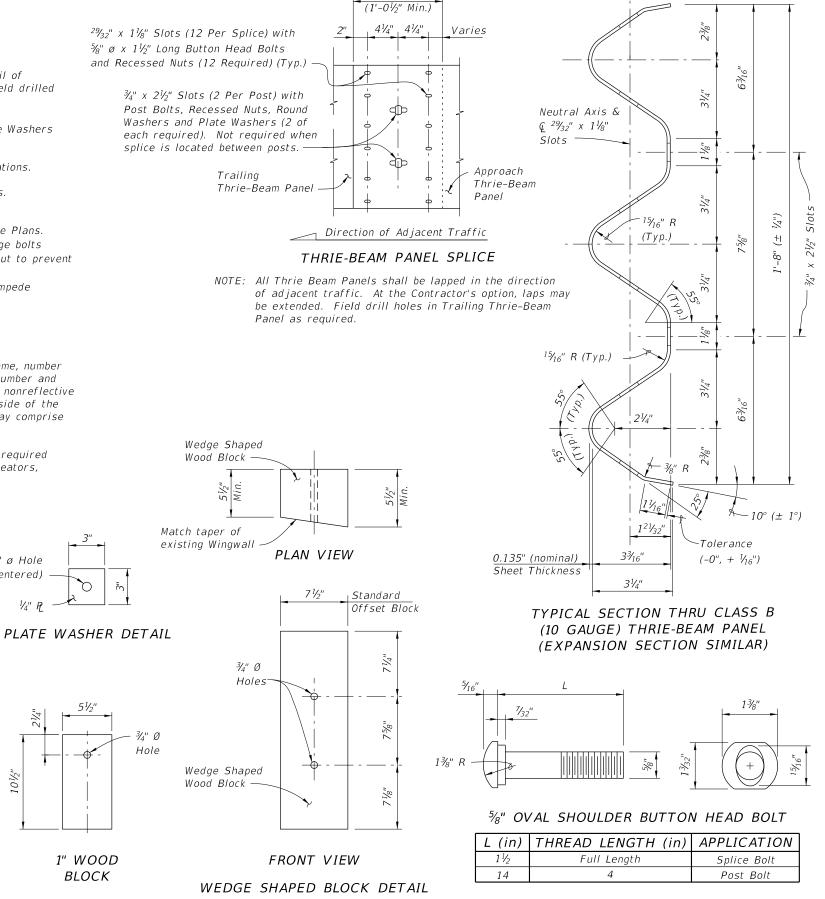
BRIDGES ON CURVED ALIGNMENTS: The details presented herein are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie-beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten bolts in 3¾" slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

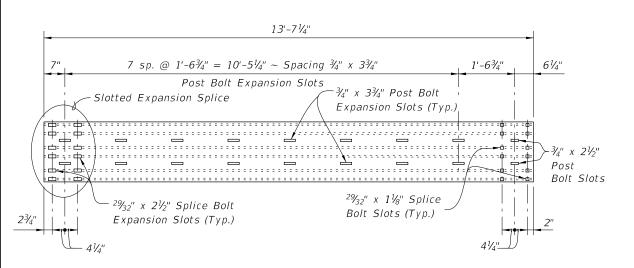
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered ($\pm \frac{1}{4}$ ").

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie-Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent quardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

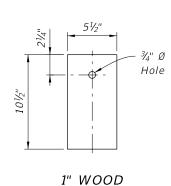
PAYMENT: Payment will be made under Thrie-Beam Panel Retrofit which shall include all materials and labor required to fabricate and install the retrofit railing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators, where required, will not be paid for directly but shall be considered incidental work.



Lap



THRIE-BEAM EXPANSION SECTION



¾" ø Hole

(centered)

REVISION 07/01/14

FDOT

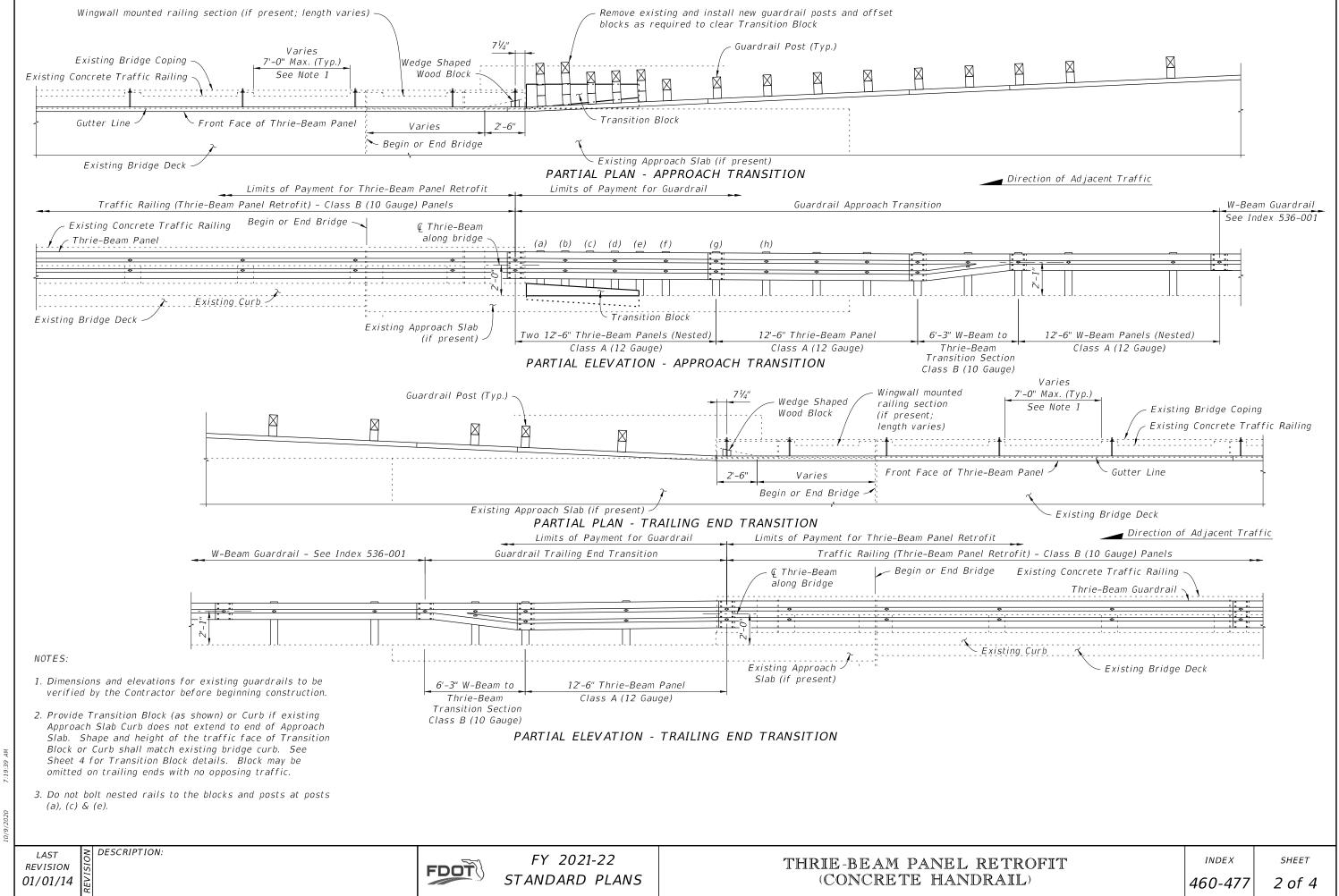
FY 2021-22 STANDARD PLANS

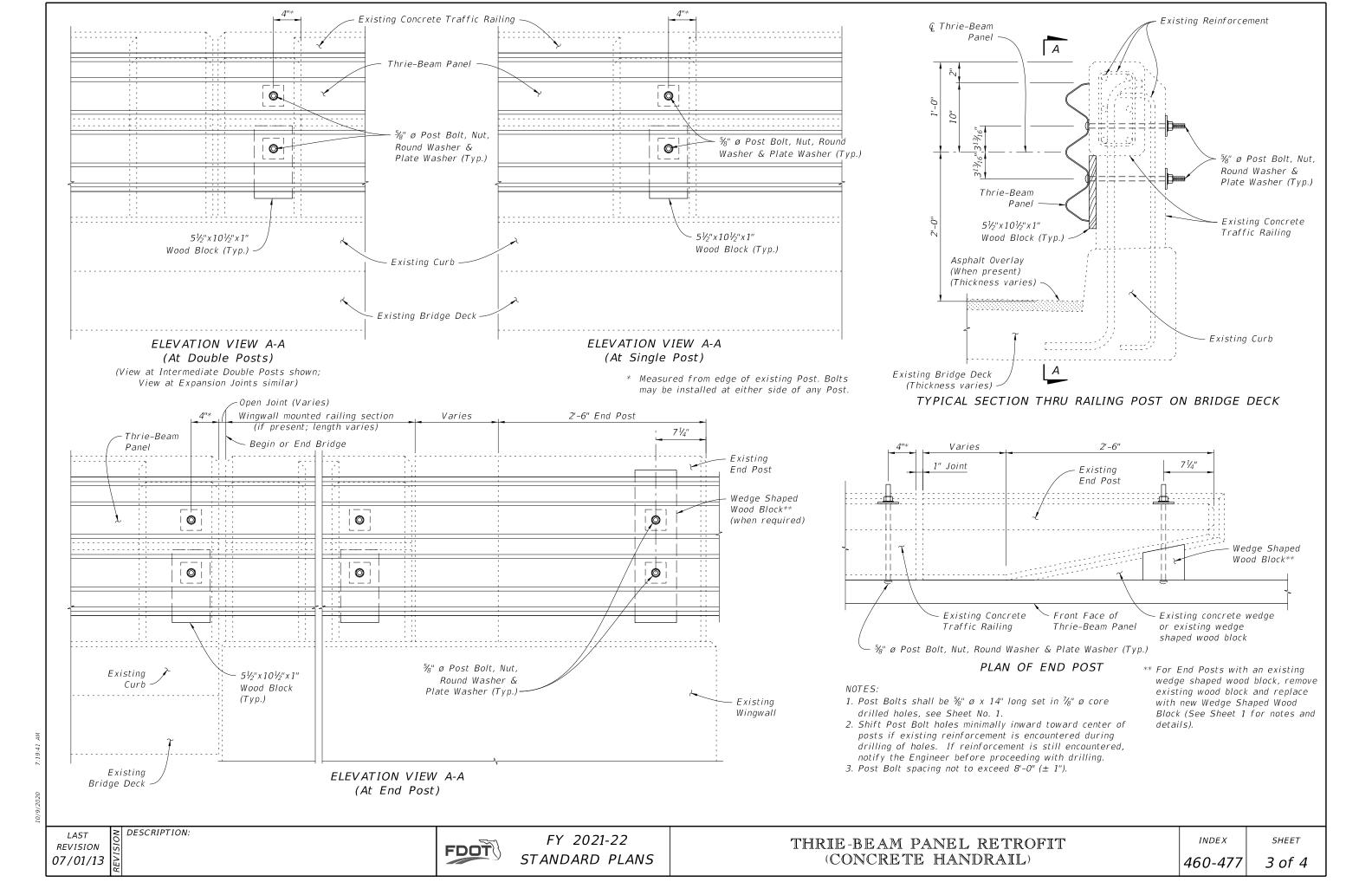
THRIE-BEAM PANEL RETROFIT (CONCRETE HANDRAIL)

INDEX 460-477

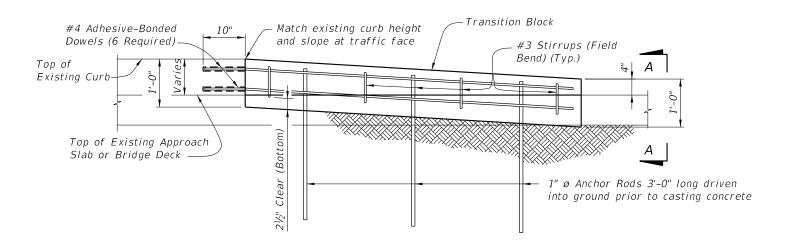
SHEET 1 of 4

DESCRIPTION:



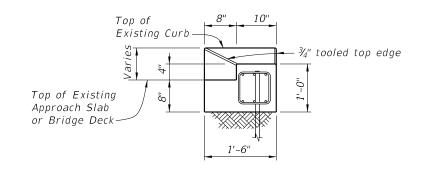


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

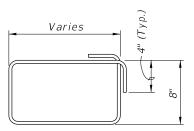


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK						
ITEM	UNIT QUANTITY					
Concrete Class II (Bridge Deck)	CY	0.4				
Reinforcing Steel	LB	61				
Guardrail (Reset)	LF	12.5				



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

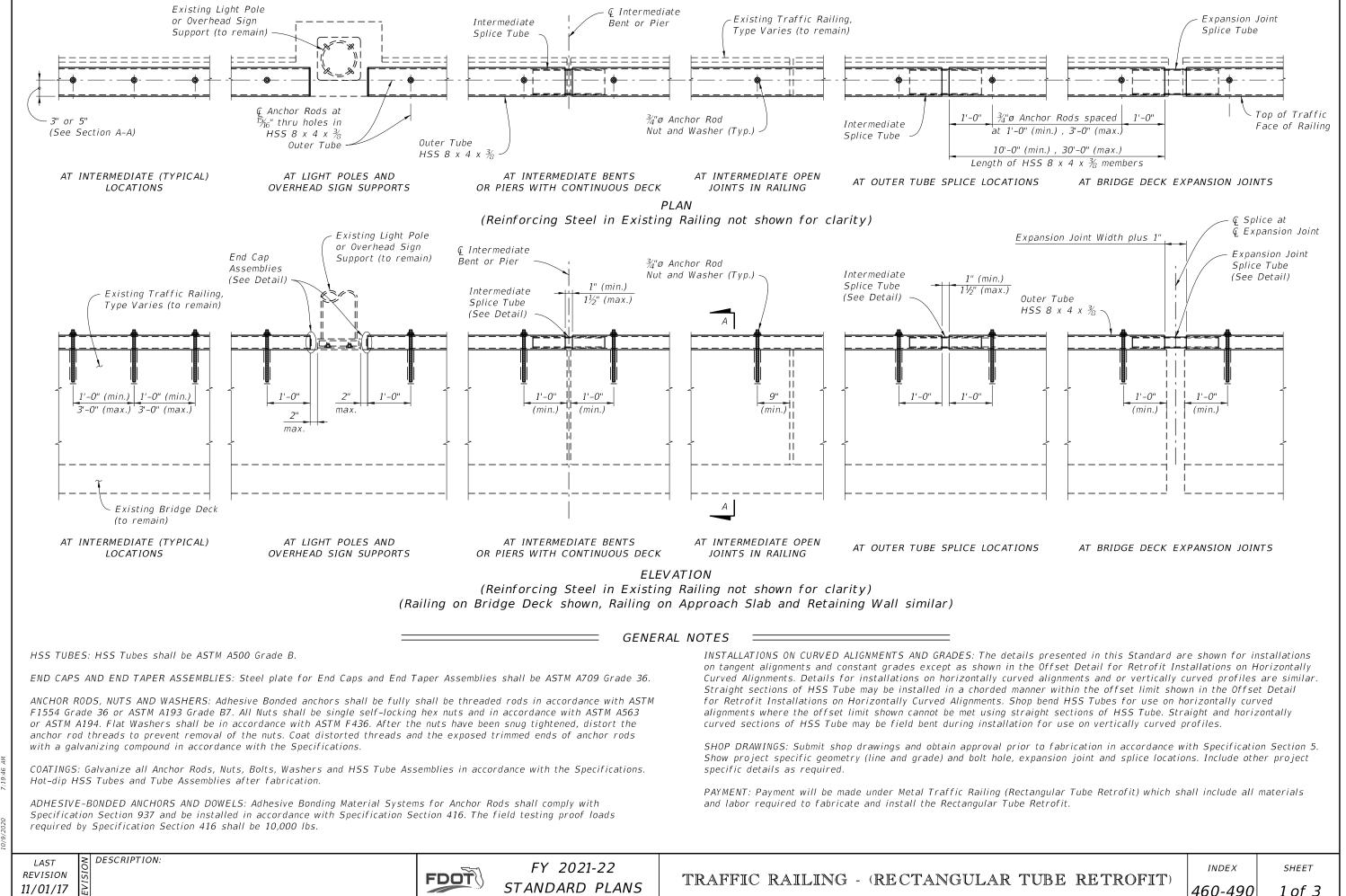
ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

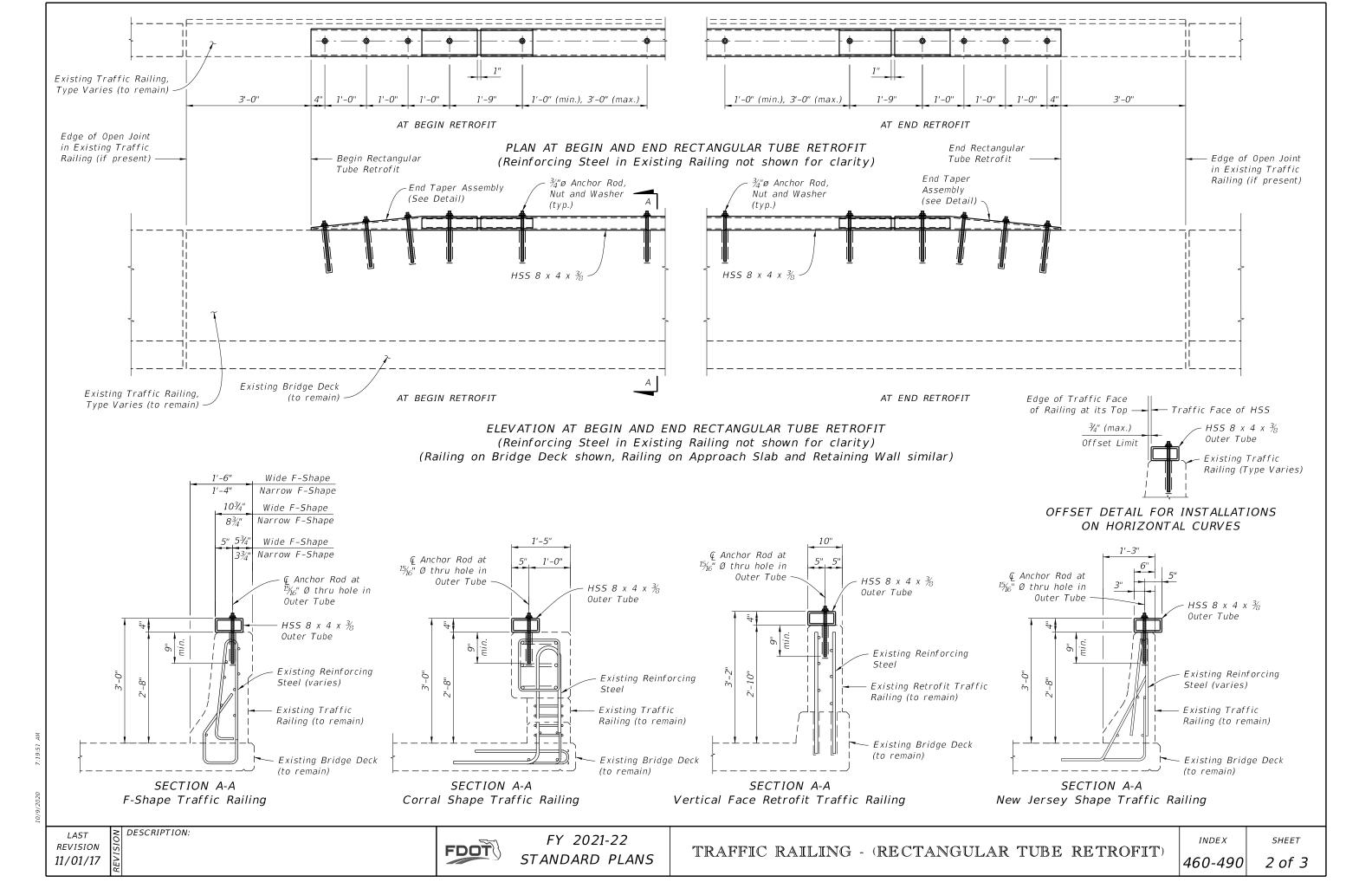
ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 (Type HV) and be installed in accordance with Specification Section 416.

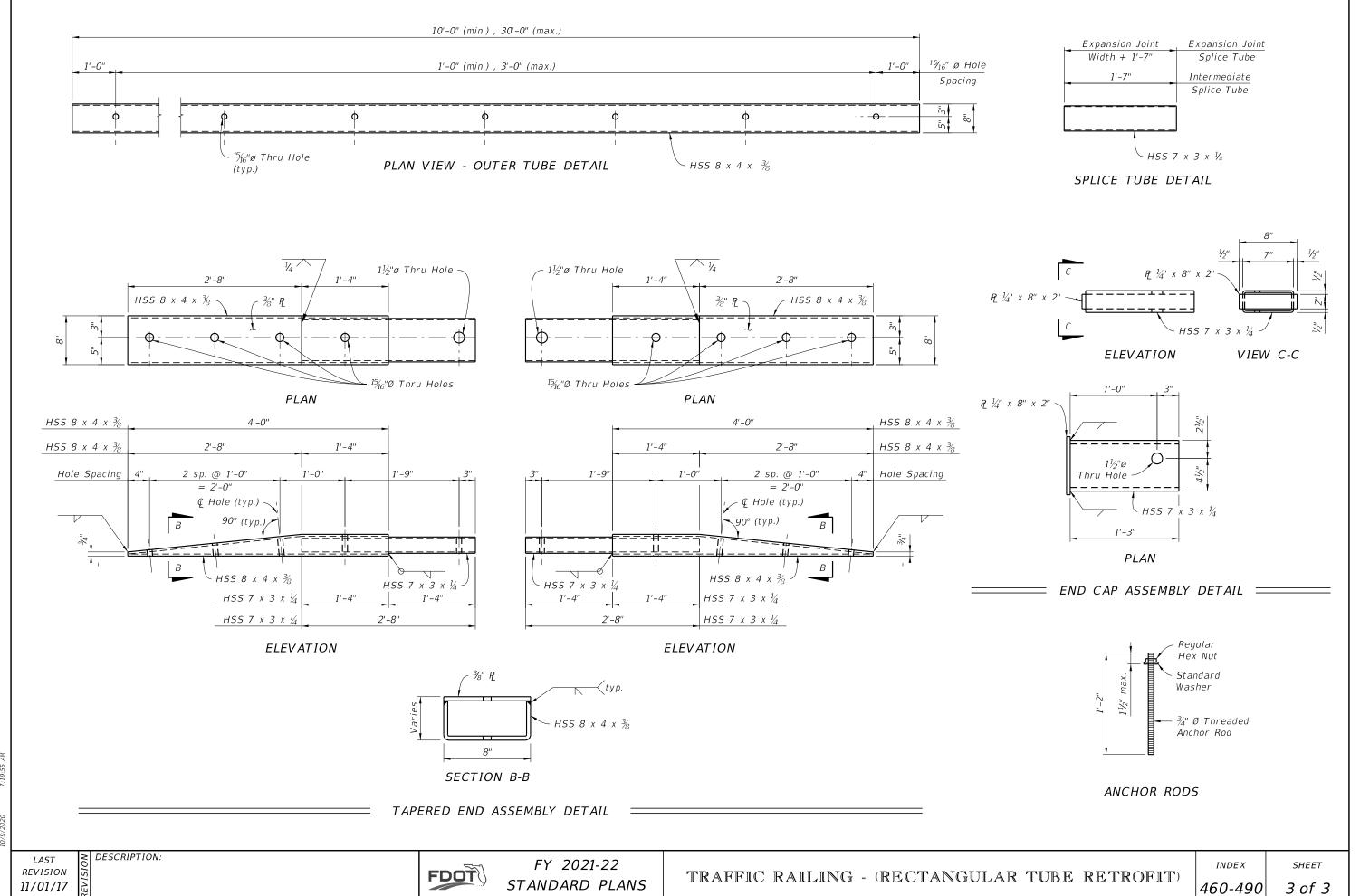
Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

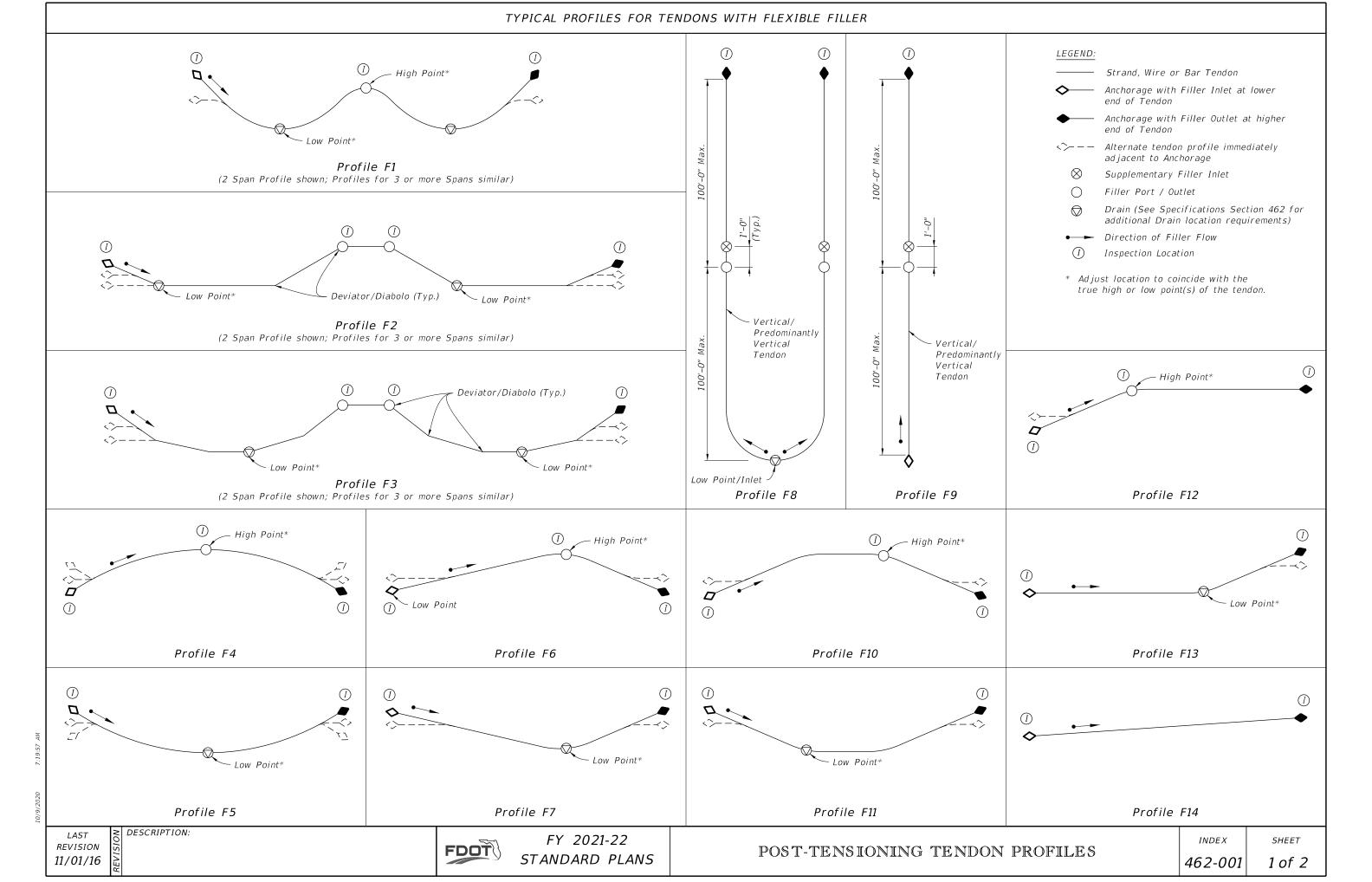
Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

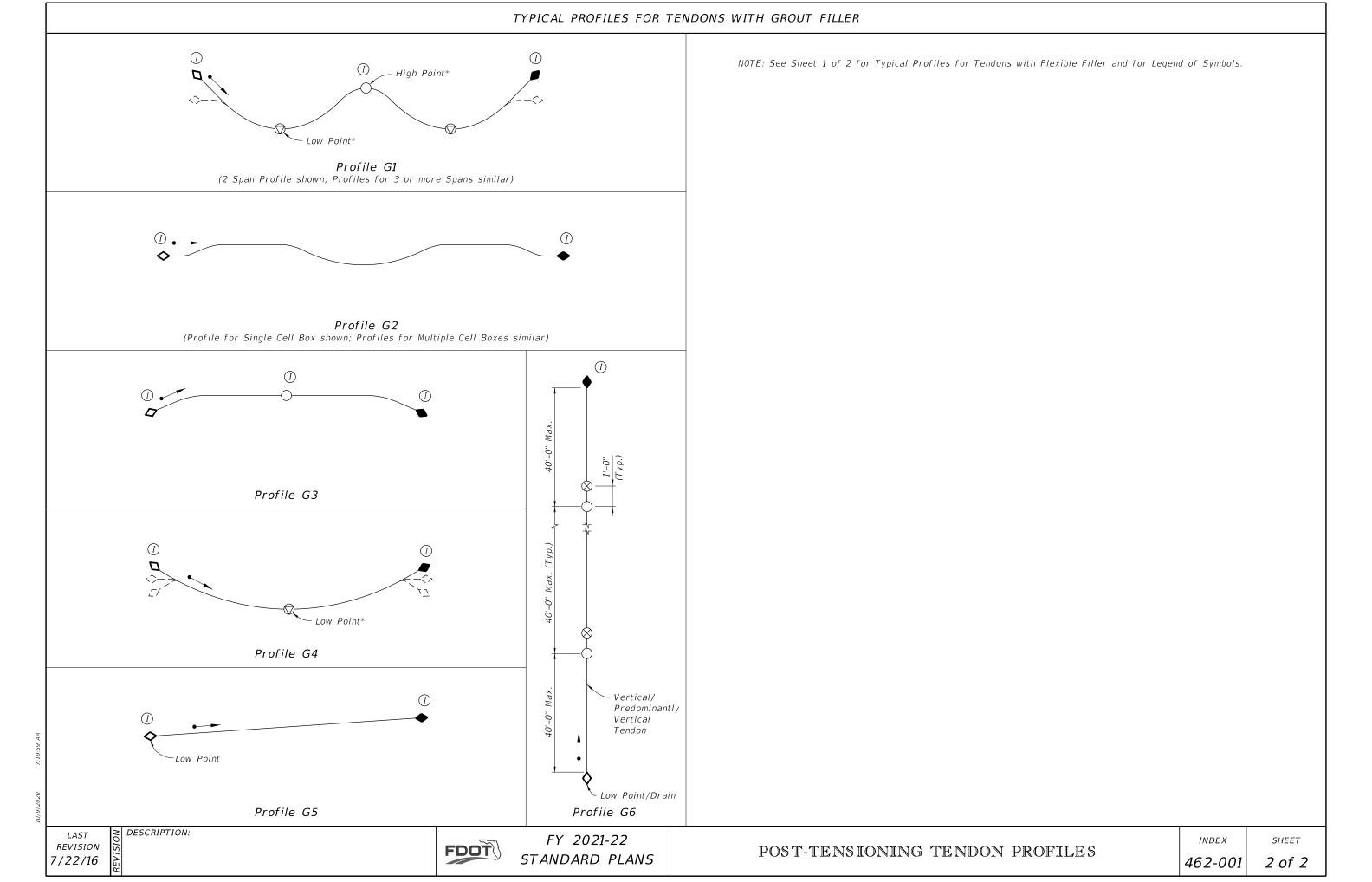
DESCRIPTION:

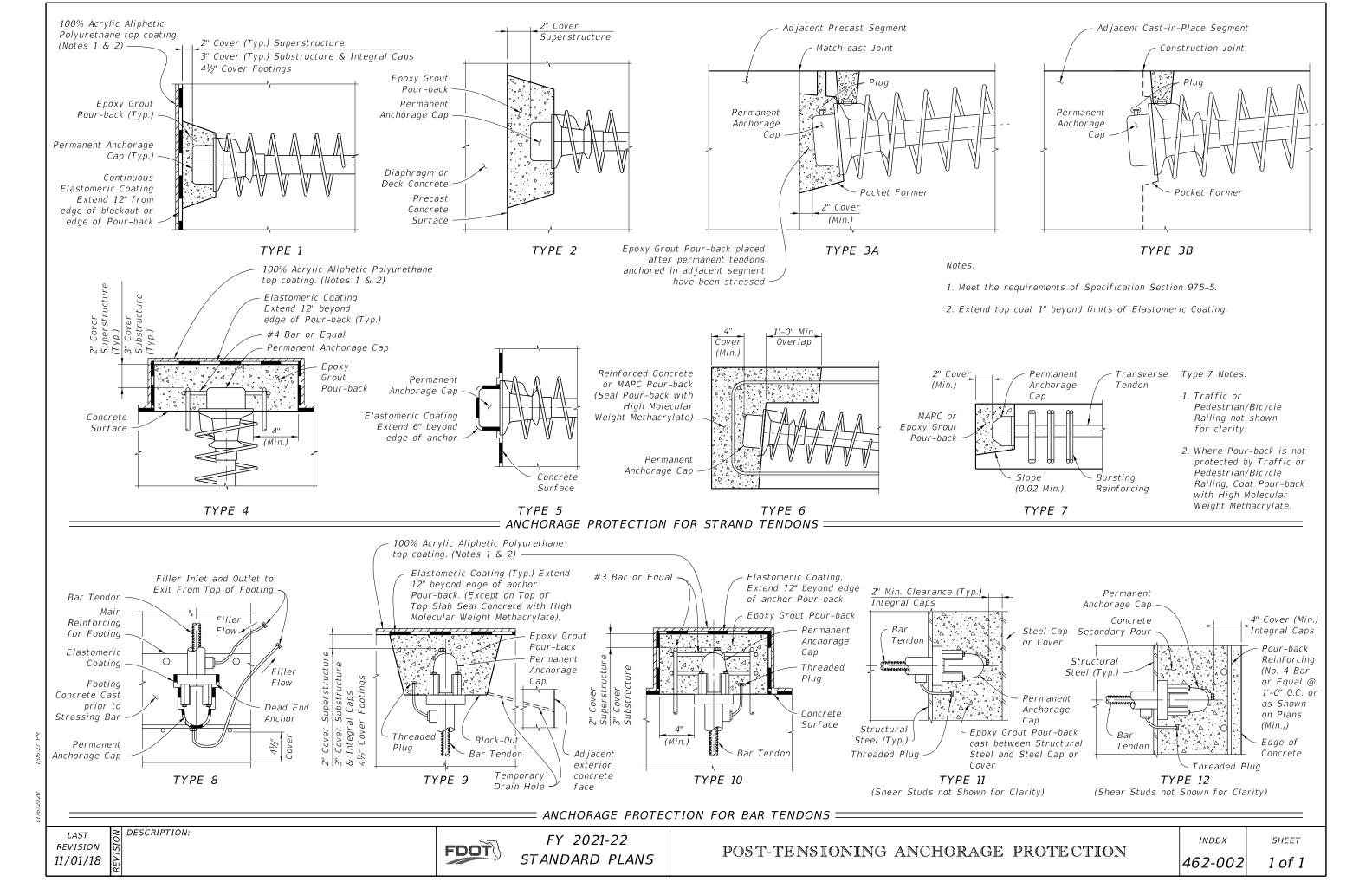


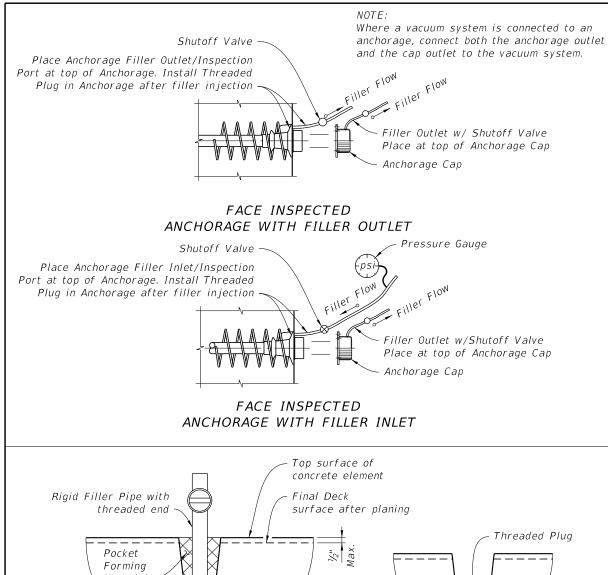


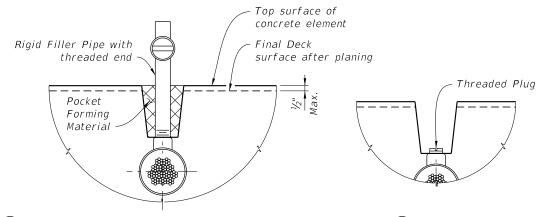


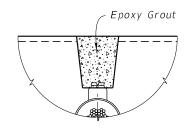












FILLER OUTLET CONNECTION TO DUCT

PROCEDURE:

1. After filler injection is completed, Remove Pocket Forming Material and Rigid Filler Pipe.

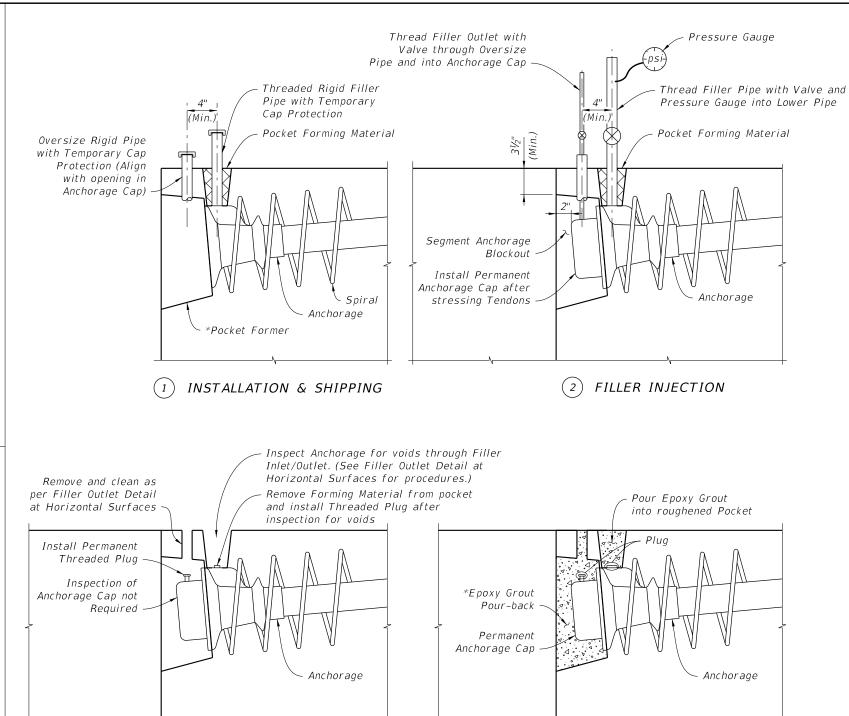
(2)POCKET

PREPARATION

- 2. Inspect Tendon for voids as necessary.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Clean threads and rethread as required.
- 5. Install Threaded Plug into Outlet to form a tight fit.
- 6. Clean and roughen sides of pocket.
- 7. Fill Pocket with Epoxy Grout.

(3) FILLING POCKET

= FILLER OUTLET DETAIL AT HORIZONTAL SURFACES =



TOP INSPECTED ANCHORAGE WITH FILLER INLET INSTALLATION, FILLER INJECTION. INSPECTION & PROTECTION

NOTES:

1. Holes used for the Inspection and Filler Inlets/Outlets may be formed using tapered pipes or mandrels.

(3) INSPECTION

- 2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.
- * Round () Pocket Former Gravity fed placement of epoxy grout acceptable Modified Square Pocket Former - Gravity fed placement of epoxy grout acceptable Square Pocket Former - Vacuum epoxy grouting required

(4) PROTECTION

DESCRIPTION:



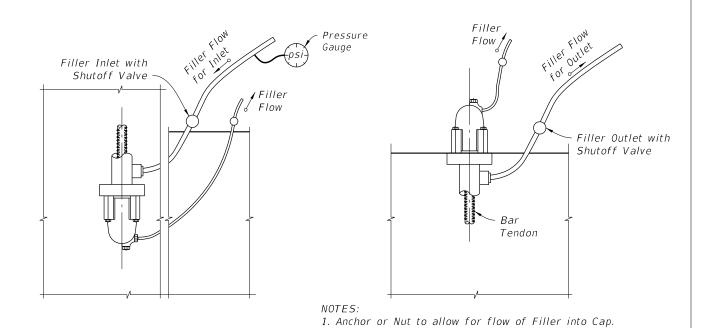
FY 2021-22

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

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INLET END

(EMBEDDED ANCHORAGE SHOWN; ANCHORAGE AT CONCRETE SURFACE SIMILAR)

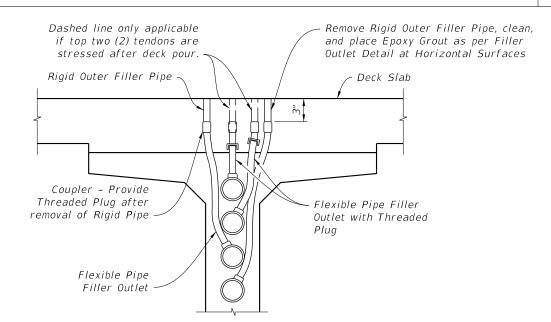
connect both the anchorage outlet and the cap outlet to the vacuum system.

OUTLET END

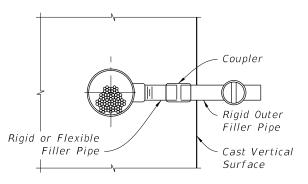
2. Where a vacuum system is connected to an anchorage,

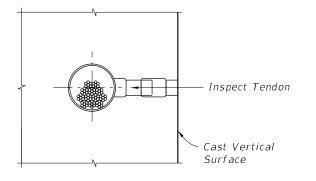
FILLER INLET AND OUTLET DETAILS FOR BAR TENDONS

(VERTICALLY ORIENTED TENDON SHOWN; HORIZONTALLY ORIENTED TENDON SIMILAR)



TENDONS AT HIGH POINTS AND 3' FROM HIGH POINTS (FILLER OUTLET)





(1) FILLER OUTLET CONNECTION TO TENDON

Epoxy Grout Threaded Plug

(3) FILLING POCKET

(2) POCKET PREPARATION

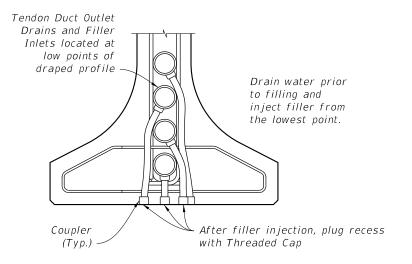
PROCEDURE:

- 1. Remove Rigid Filler Pipe or drill Grout in flexible pipe.
- 2. Inspect tendon for voids.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Install Threaded Plug into Outlet to form a tight fit.
- 5. Over-ream hole ($\frac{1}{4}$ " Ø over-ream). Clean and roughen sides.
- 6. Fill pocket with epoxy grout.

FILLER OUTLET DETAIL AT VERTICAL SURFACES

Cast Vertical

Surface



TENDONS AT LOW POINTS (FILLER INLET / DRAIN)

== FILLER INLET AND OUTLET DETAILS FOR I-GIRDERS ======

DETAILS FOR C.I.P. BOXES WITH INTERNAL TENDONS SIMILAR, WEB REINFORCING NOT SHOWN FOR CLARITY.

REVISION 11/01/16

DESCRIPTION:

FDOT

FY 2021-22 STANDARD PLANS

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

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462-003

- U.S. COAST GUARD NOTIFICATION: Notify the local office of the U.S. Coast Guard at least 30 days prior to beginning of construction of the Fender System.
- 14" SQUARE PRESTRESSED CONCRETE PILES Provide 14" Square Prestressed Concrete Piles of sufficient length to achieve a minimum embedment of 20' into soil having a blow count greater than or equal to 6 ($N \ge 6$). Pile splices and build-ups are not permitted. Use only 14" Square Prestressed Concrete Piles with 8 $\frac{1}{2}$ " diameter Low Relaxation Strands fabricated in accordance with Index 455–014.
- PLASTIC LUMBER AND STRUCTURAL COMPOSITE LUMBER WALES: Provide only Plastic Lumber (Thermoplastic Structural Shapes) and Structural Composite Lumber (Reinforced Thermoplastic Structural Shapes) Wales in accordance with Specification Section 973. Wales shall be continuous and spliced only at locations shown on the plans.
- PLASTIC LUMBER DECKING FOR CATWALKS: Provide Plastic Lumber decking for catwalks when called for in the Plans in accordance with Specification Section 973.

Install Plastic Lumber Decking according to manufacturer's recommendations using stainless steel $\#10 \times 3$ " (minimum) deck screws.

FIBERGLASS OPEN GRATING FOR CATWALKS: Provide Fiberglass Open Grating for catwalks when called for in the Plans. Fiberglass Open Grating shall be a heavy duty design suitable for exterior installations. Maximum gap opening on the walkway surface shall be $1\frac{1}{2}$ ". Design live loads and deflections shall be a 50 psf uniformly distributed load with a maximum deflection of $\frac{3}{8}$ " or L/120 at the center of a simple span and a concentrated load of 250 pounds with a maximum deflection of $\frac{1}{4}$ " at the center of a simple span. Color of Fiberglass Open Grating shall be gray or black.

Install Fiberglass Open Grating according to manufacturer's recommendations using stainless steel hardware, screws, bolts, nuts and washers. Attach Fiberglass Open Grating to Wales and Deck Supports at a 2'-0" maximum spacing so as to resist pedestrian live loads and uplift forces from wind, buoyancy and wave action.

- CLEARANCE GAUGE AND LIGHT: Clearance Gauge to be furnished and installed by the Contractor. Clearance Gauge width and numeral height is dependant on visibility distance. The required visibility distance shall be determined by the United States Coast Guard District Commander. Provide and install Clearance Gauge Light in accordance with Specification Section 510 and Index 510-001.
- NAVIGATION LIGHTS: Provide and install Navigation Lights in accordance with Specification Section 510, Index 510-001 and/or project specific details. Provide and maintain Temporary Navigation Lights during construction until permanent Navigation Lights are operational.
- BOLTS, THREADED BARS, NUTS, SCREWS AND WASHERS: Furnish stainless steel Bolts in accordance with ASTM F593 Type 316. Furnish stainless steel Threaded Bars in accordance with ASTM A193 Grade B8M. Furnish stainless steel Nuts in accordance with ASTM F594 Type 316. Furnish stainless steel Screws in accordance with ASTM F593 Type 305. Furnish stainless steel Washers compatible with Bolts, Threaded Rods and Nuts under heads and nuts. Torque Nuts on 1" diameter Bolts and Threaded Bars to 150 lb-ft. Keep threads on Bolts, Threaded Bars and Nuts free from dirt, coarse grime and sand to prevent galling and seizing during tightening.

SPLICE PLATES: Furnish Splice Plates in accordance with ASTM A240 Type 316.

WIRE ROPE: Provide wire rope meeting one of the following requirements:

- 1. $\frac{1}{2}$ " diameter 6x19, 6x25 or 6x37 class IWRC Type 316 stainless steel wire rope with a minimum breaking strength of 18.000 lbs.
- 2. ½" diameter 6x19 galvanized wire rope with ultraviolet ray resistant polypropylene impregnation having an outside diameter of 5/8" with a minimum breaking strength of 22,000 lbs. Protect all ends with heat shrinkable end caps compatible with the rope's polypropylene that provide an effective water-tight seal.

FENDER SYSTEM ENERGY CAPACITY: Energy Capacity = 38 ft-k

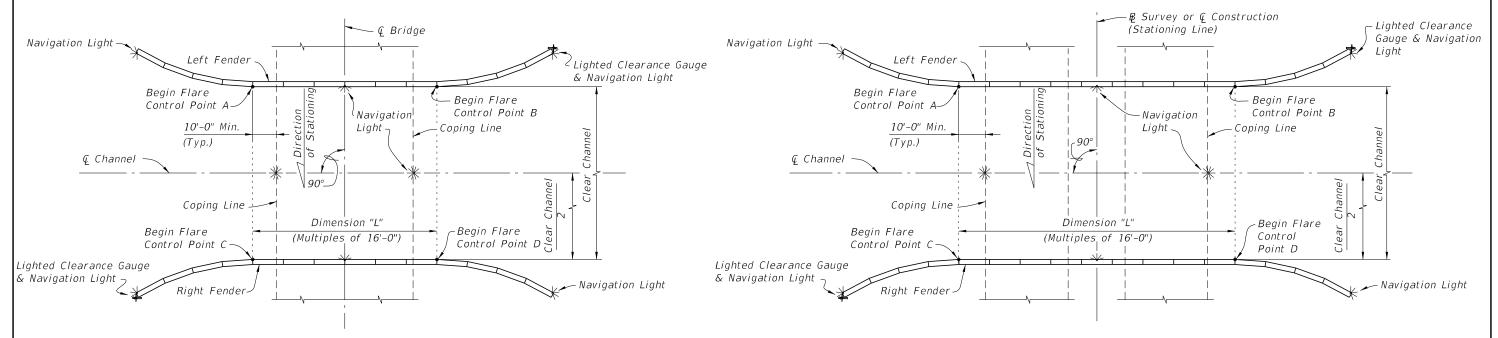
GENERAL NOTES

11/6/2020

LAST REVISION 07/01/14

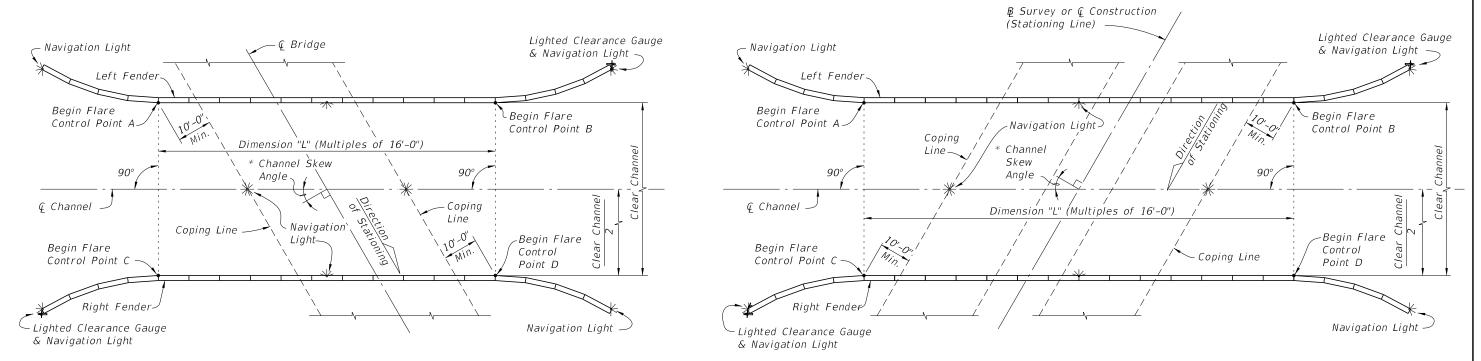
DESCRIPTION:

FDOT



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH NONSKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH NONSKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH SKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH SKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)

* See Structures Plans, Plan and Elevation and Foundation Layout Sheets for magnitude and orientation of Channel Skew Angle. CROSS REFERENCES:

For Stations and Offsets of referenced Control Points A, B, C and D, Dimension "L" and Clear Channel Width see Fender System Table of Variables in Structures Plans.

For Navigation Light Details see Design Standards Index 510-001.

LAYOUT GEOMETRY

LAST REVISION 07/01/11

DESCRIPTION:

FDOT

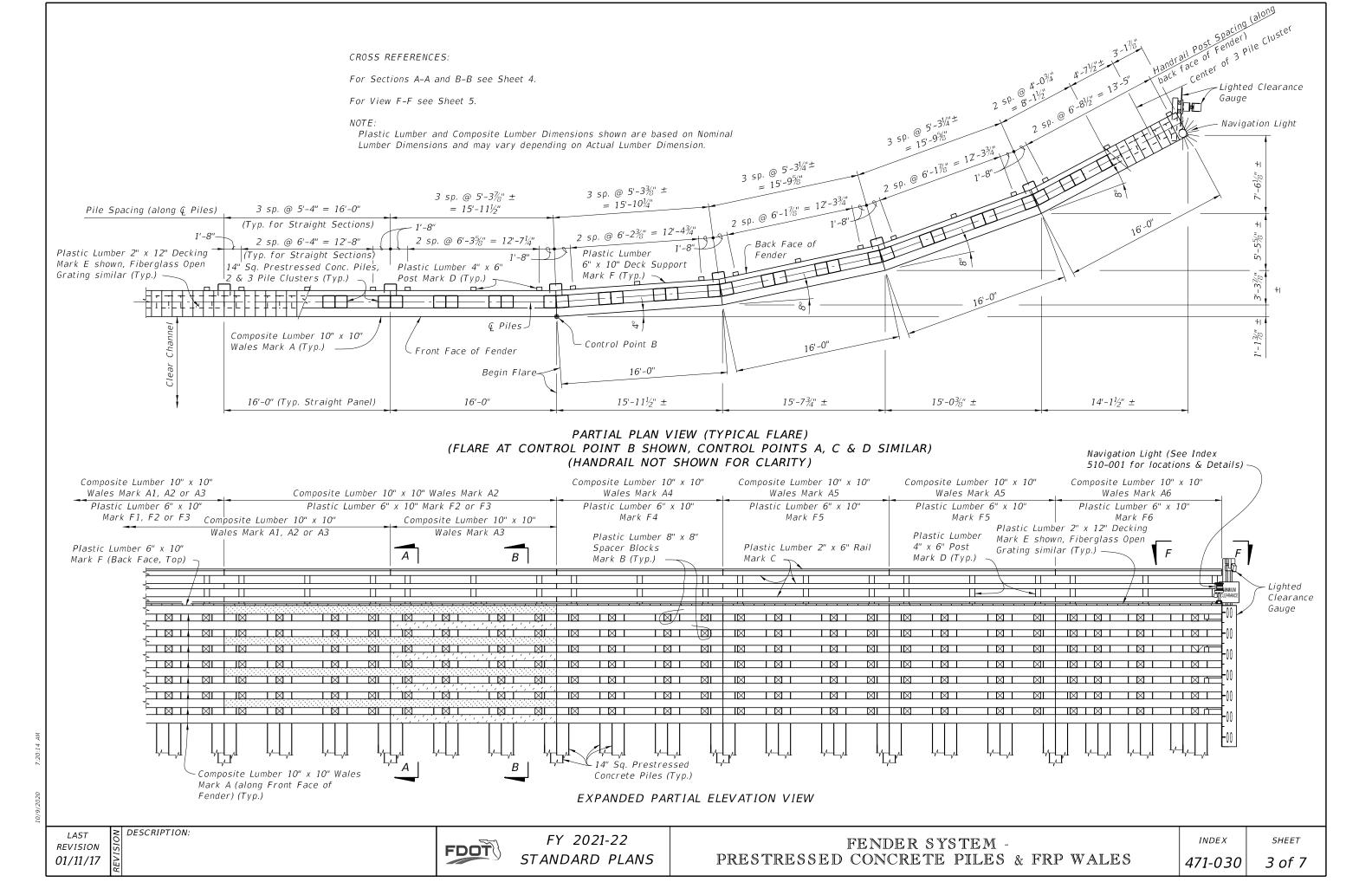
FY 2021-22 STANDARD PLANS

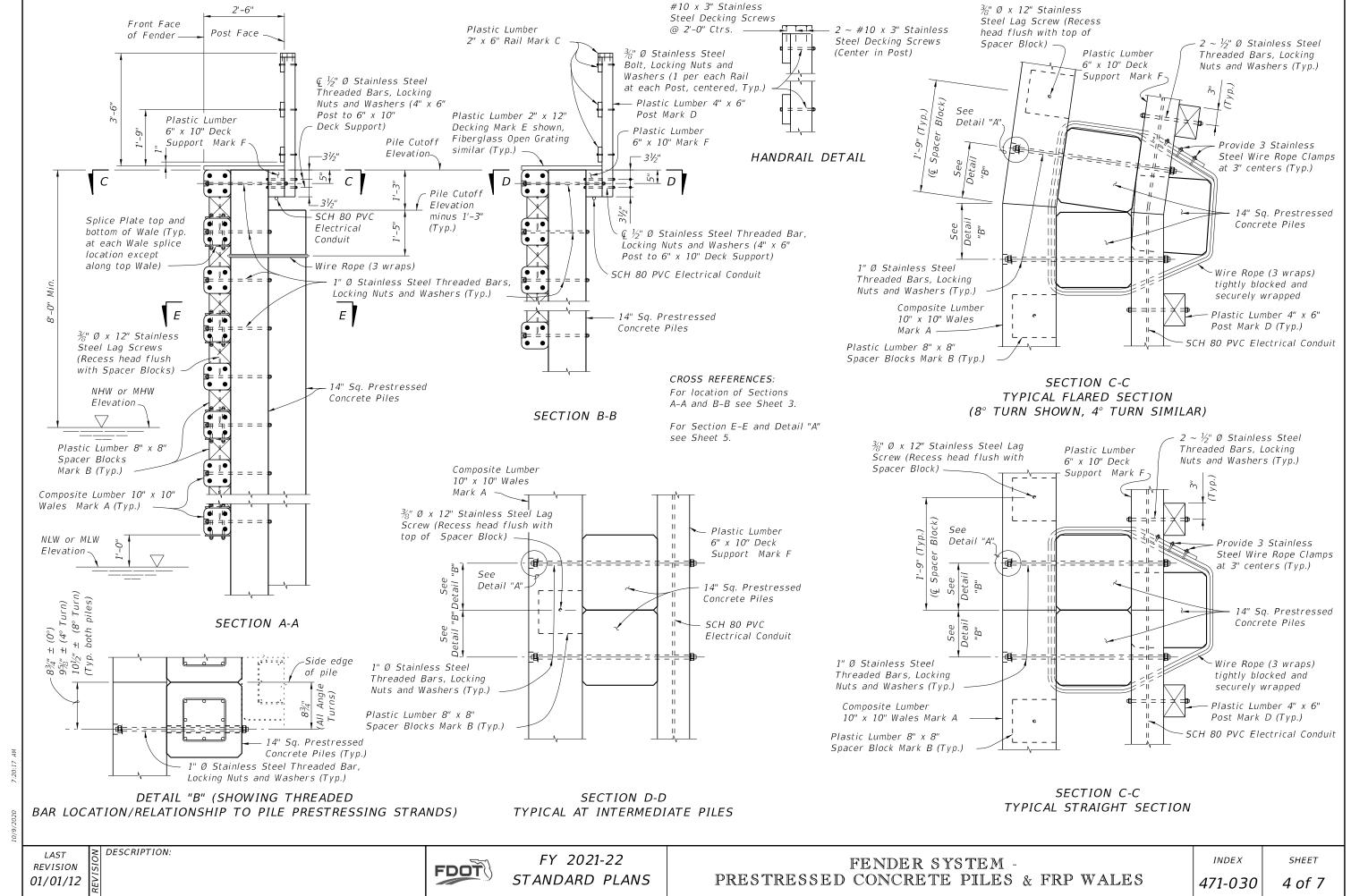
FENDER SYSTEM - PRESTRESSED CONCRETE PILES & FRP WALES

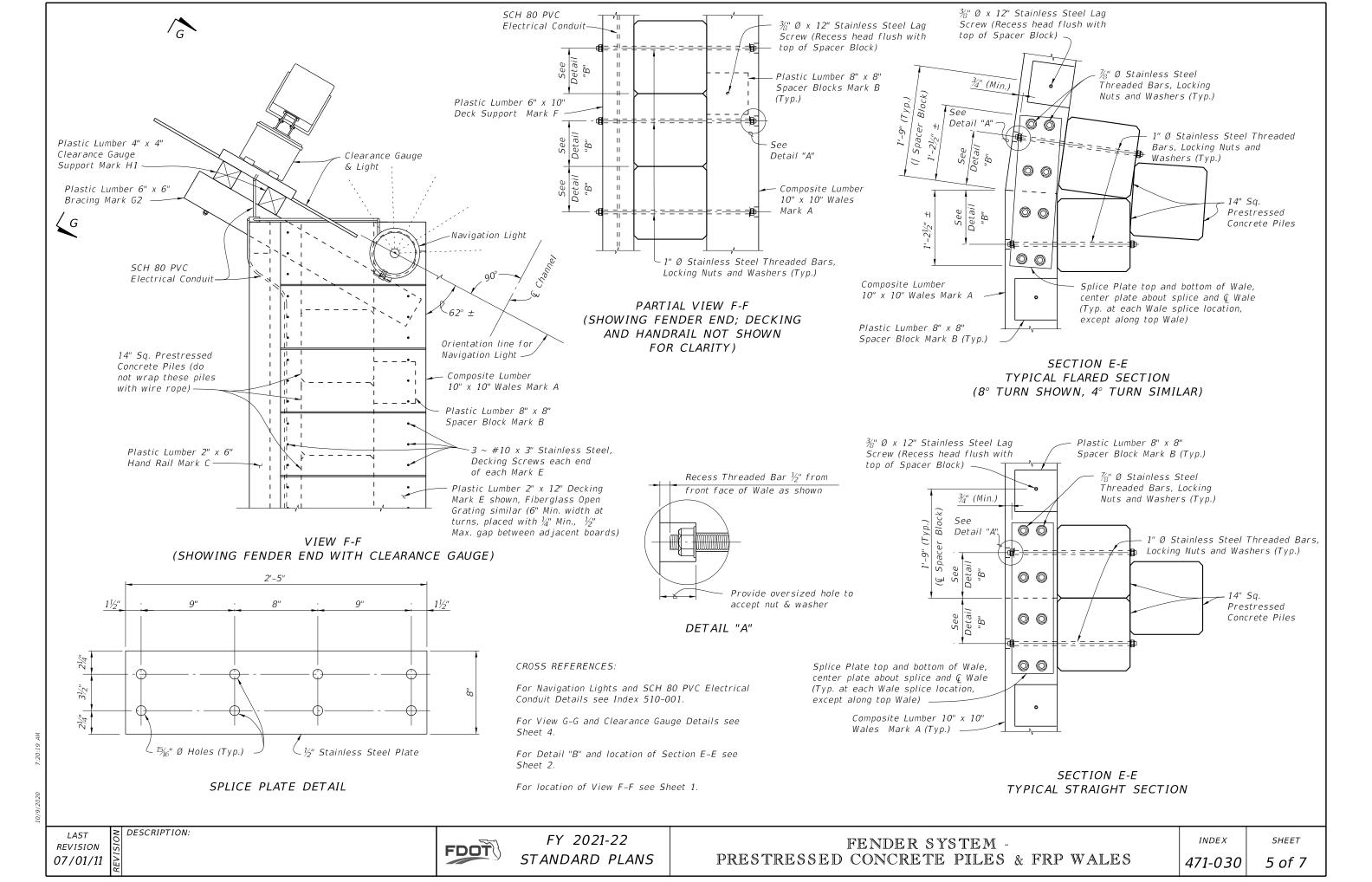
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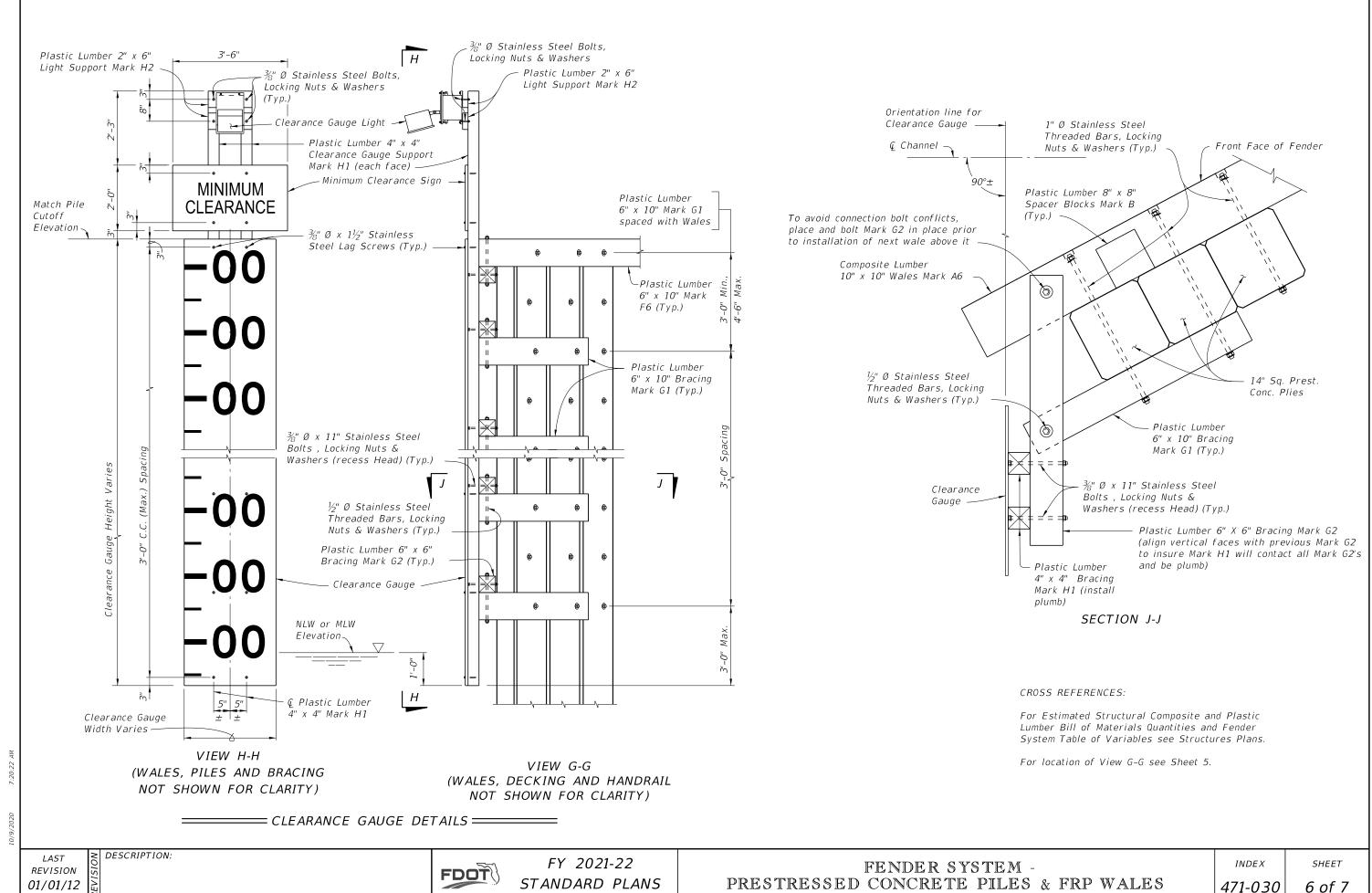
SHEET

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* STRUCTURAL COMPOSITE LUMBER BILL OF MATERIALS								
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY			
A1	10" X 10" COMPOSITE LUMBER	32'-0" (STRAIGHT)	266.6	nber				
A2	10" X 10" COMPOSITE LUMBER	32'-0"	266.6	and Plastic Lumber	res Plans			
АЗ	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	mposite and Pla				
A4	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	tructural Co	Materials Table			
A5	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	Estimatee Bill of		of		
A6	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	See				

	* PLASTIC LUMBER BILL OF MATERIALS								
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY				
В	8" X 8" PLASTIC LUMBER	8" (STRAIGHT)	3.6	-					
С	2" X 6" PLASTIC LUMBER	16'-0" (STRAIGHT) (Trim & Miter Ends as required)	16.0						
D	4" X 6" PLASTIC LUMBER	4'-4" (STRAIGHT)	8.7						
** E	2" X 12" PLASTIC LUMBER	2'-6" (STRAIGHT) (Miter as required, 6" Min. width)	5.0	Estimated Structural Composite and Plastic Lumber Bill of Materials Table in Structures Plans					
F 1	6" X 10" PLASTIC LUMBER	32'-0" (STRAIGHT)	160.0						
F2	6" X 10" PLASTIC LUMBER	31'-11"	159.6						
F3	6" X 10" PLASTIC LUMBER	15'-11"	79.6						
F4	6" X 10" PLASTIC LUMBER	15'-91/4"	78.8						
F5	6" X 10" PLASTIC LUMBER	15'-81/4"	78.4	See Estimated S Bill of Ma					
F6	6" X 10" PLASTIC LUMBER	15'-101/4"	79.3						
G 1	6" X 10" PLASTIC LUMBER	3'-8" (STRAIGHT)	18.3						
G2	6" X 6" PLASTIC LUMBER	4'-1" (STRAIGHT)	12.3						
Н1	4" X 4" PLASTIC LUMBER	PILE CUTOFF ELEV. MINUS NLW OR MLW ELEV. PLUS 5'-6" (STRAIGHT)	1.3 PER LF EACH						
Н2	2" X 6" PLASTIC LUMBER	1'-2" (STRAIGHT)	1.2						

^{*} All Plastic Lumber and Composite Lumber Dimensions and Quantities shown are based on Nominal Lumber Dimensions and may vary depending on Actual Lumber Dimension.

^{**} Provide Fiberglass Open Grating in lieu of 2" X 12" Plastic Lumber when called for in the Plans. Mounting hardware shall be Stainless Steel, install per Manufacturer's recommendations. See Structures Plans for Notes and Details.