

FY 2020-21 STANDARD PLANS FOR BRIDGE CONSTRUCTION

Effective for Projects with Lettings in the Fiscal Year (FY) from July 1, 2020 through June 30, 2021

FY 2019-20 Standard Plans for Road and Bridge Construction Topic No. 625-010-003 State of Florida Department of Transportation Office of Design Mail Station 32 605 Suwannee Street Tallahassee, Florida 32399-0450

FDOT FY 2 0 2 0 - 2 1 STANDARD PLANS

NOTICE

The Standard Plans are intended to support the various engineering processes for construction operations on the State Highway System. They are established to ensure the application of uniform standards in the preparation of contract plans for construction of roadways and structures. These Standard Plans may be used for maintenance operations or adopted by other authorities for use on projects under their jurisdiction.

It is the responsibility of the Engineer of Record using these Standard Plans to determine the fitness for a particular use of each standard in the design of a project. The inappropriate use of and adherence to these standard Plans does not exempt the engineer from the professional responsibility of developing an appropriate design.

PATENTED DEVICES, MATERIALS AND PROCESSES

The use of any design, method, process, material or device either expressed or implied by these standards that are covered by patent, copyright, or proprietary privilege is the sole responsibility of the user. Any infringement on the rights of the inventor, patentee, assignee or licensee shall be the sole responsibility of the user. For additional information refer to Subsection 7-3 of the FDOT Standard Specifications for Road and Bridge Construction.

DISTRIBUTION OF EXEMPT PUBLIC DOCUMENTS:

It is the policy of the Department to protect the State Highway System's infrastructure by defining the responsibilities for disclosure and use of sensitive documents showing the structural elements used in the design and construction of Department structures. Section 119.071(3)(b), Florida Statute (F.S.), provides that these sensitive documents are exempt from Chapter 119, F.S., Florida's public records law. In accordance with Section 119.071(3)(b), F.S., the Department has adopted Procedure 050-020-026, Distribution of Exempt Public Documents Concerning Department Structures and Security System Plans, to define the method and responsibilities for disclosure and use of these sensitive documents.

Structure is defined in Section 334.03(27), F.S., as "a bridge, viaduct, tunnel, causeway, approach, ferry slip, culvert, toll plaza, gate, or other similar facility used in connection with a transportation facility" which would include related pipes and pipe systems. However, for the purpose of the public records law and Procedure 050–020–026, the Department has determined that the term "structure" includes "bridges with an opening of more than 20 feet between undercopings of abutments or spring lines of arches or extreme ends of openings for multiple boxes, and those other bridges subject to safety inspection under Section 335.074, F.S." A roadway is not otherwise a structure for the purposes of Procedure 050–020–026.

Therefore, plans, blueprints, schematic drawings, and diagrams of structures owned by the Department are exempt from the public records provisions of Chapter 119, F.S. This exemption includes draft, preliminary, and final formats as described in Procedure 050-020-026 and includes paper, electronic, and other formats. The Department has provided for the limited release of such documents in Procedure 050-020-026.

Entities or persons outside the Department requesting or receiving copies of any portion of plans or other documents considered Exempt Documents under Procedure 050-020-026 must complete and submit a request form (Form No. 050-020-26). The form also advises the requestor that the entity or person receiving the documents shall maintain their exempt status. This procedure applies to all Department internal or contracted staff who have access to such Exempt Documents in their Department work. Refer to Procedure 050-020-026 for additional requirements.

CERTIFICATION STATEMENT

I hereby certify that these Standard Plans were prepared by me or under my responsible charge, compiled from designs prepared, examined, adopted and implemented by the Florida Department of Transportation in accordance with established procedures, and as approved by the Federal Highway Administration.

Derwood C. Sheppard, Jr. M.Eng., P.E. State Standard Plans Engineer

The official version of the Standard Plans is the PDF version and can be found at:

http://www.fdot.gov/design/standardplans



Florida Department of Transportation

RON DESANTIS GOVERNOR

605 Suwannee Street Tallahassee, FL 32399-0450 KEVIN J. THIBAULT, P.E. SECRETARY

October 24, 2019

James Christian
Division Administrator
Federal Highway Administration
3500 Financial Plaza, Suite 400
Tallahassee, Florida 32312

Re:

Office of Design

FY 2020-21 Standard Plans for Road and Bridge Construction

Dear Mr. Christian:

In accordance with the Stewardship and Oversight Agreement on Project Assumption and Program Oversight by and between the Federal Highway Administration, Florida Division, and the State of Florida Department of Transportation the Department has provided the FY 2020-21 Standard Plans for Road and Bridge Construction (Standard Plans) for review. Copies of all revised Indexes for the FY 2020-21 Standard Plans were provided to the Florida Division Office in three different submittal packages between August 22nd and September 19th and all comments have been addressed to the satisfaction of the reviewer(s). Consequently, the Department is requesting approval of the FY 2020-21 Standard Plans for use on federal-aid projects.

Sincerely,

Derwood Sheppard, P.E.

State Standard Plans Engineer

For FHWA Florida Division Office use:

Approved for Use on Federal Aid Projects:

Bren Greorge-Nwabugwu Sr. 10/28/19
James Christian, P.E.

For

Division Administrator

www.fdot.gov

ABBREVIATIONS

FY 2020-21 STANDARD PLANS

			0-21 STANDARD PLANS	Abbasslatt	Manufac
Abbreviation	<u>Meaning</u>	Abbreviation		Abbreviation	Meaning
A AASHTO	American Association Of State Highway And Transportation Officials	(CP	Concrete Pipe	G	Shear Modulus
AC	Alternating Current	CSIP	Cost Savings Initiative Proposal	g	Gram
Accel.	Acceleration	CSL	Cross-hole Sonic Logging	Ga.	Gauge or Gage
ACI	American Concrete Institute	СТРВ	Cement Treated Permeable Base	Galv.	Galvanized
ADA	Americans With Disabilities Act	Ctr., Ctrs.	Center	GFI	Ground Fault Interrupter
ADT	Average Daily Traffic	Cu. Ft.	Cubic Feet	GFRP	Glass Fiber Reinforced Polymer
AFAD	Automated Flagger Assistance Device	Cu. Yd., CY,	Cubic Yard	Grd.	Ground
AISC	American Institute Of Steel Construction		D		H
AISI	American Iron and Steel Institute	D	Depth, Distance or Diameter	Hd.	Head
Alt.	Alternate	Dia. or Ø	Diameter	H.S., HS	High Strength
Alum.	Aluminum	Dbl.	Double	HDPE	High Density Polyethylene
ANSI	American National Standards Institute	Decel.	Deceleration	Horiz.	Horizontal
A05	Apparent Opening Size	Deg.	Degree	HP	Horsepower or H-Pile
APL	Approved Products List	Dim.	Dimension	HSHV	High Strength Horizontal Vertica
App.	Approach	Dist.	Distance		I
Approx.	Approximate	DMM	Domestic Mail Manual	ID, I.D.	Inside Diameter or Identification
ARTBA	American Road & Transportation Builders Association	DPI	Ditch Point Intersection	in.	Inch(es)
Asph.	Asphalt	Dt	Ditch	Inc.	Incorporated
Assem.	Assembly	DT0E	District Traffic Operations Engineer	Int.	Interior
ASTM	American Society For Testing And Materials		E	Inv.	Invert
ATPB	Asphalt Treated Permeable Base	e	Superelevation Rate	ITS	Intelligent Transportation System
Auxil.	Auxiliary	E.P. or EOP	Edge Of Pavement		J
AWG	American Wire Gauge	EA or Ea.	Each	JCT	Junction
AW S	American Welding Society	EIA	Electronic Industries Alliance	Jt.	Joint
R		El. or Elev.	Elevation		K
Bot.	Bottom	Embed.	Embedment	k	kip
Brkwy.	Breakaway	EPDM	Ethylene Propylene Diene Monomer	kip	1000 Pounds
b/w	Between	Eq.	Equation or Equal	ksi	Kips Per Square Inch
C		Equip.	Equipment	kVA	Kilovolt Ampere
CC, C to C	Center to Center	etc.	Et Cetera (And So Forth)		1
	Curb And Gutter	ETP	Electronic Tough Pitch	1	Length
C.C.	Crash Cushion	Ex.	Example	LA	Limited Access
CCTV	Closed-Circuit Television	Exist.	Existing	lb or lbs.	Pound(s)
CFR	Code of Federal Regulations	Exp.			Pounds Per Square Yard
CFRP	Carbon Fiber Reinforced Polymer	Exp. Ext.	Expansion Extension	lb/sy lbf	Pound force
cfs, CFS	Cubic Feet Per Second	LXI.	F	LBR	Lime rock Bearing Ratio
CIP, C.I.P. or C-I-P	Cast In Place	FAC	Florida Administrative Code	LF	Linear Foot (Feet)
CJP		FC	Friction Course		Length
Ckt.	Complete Joint Penetration	Fdn.	Foundation	Lgth.	Longitudinally or Longitudinal
	Contan Line			Long.	
Q.	Clearance	F.L. or F	Flow Line	LRFD	Low Polyvation Strand
CI.	Clearance Corrugated Motal Ring	FI.	Florida Department Of Environmental Protection	LRS	Low-Relaxation Strand
CMP	Corrugated Metal Pipe Connection	FDEP FDOT	Florida Department Of Environmental Protection Florida Department Of Transportation	LS LSD	Lump Sum Lump Sum per Day
Con.		FHWA			
Conc.	Concrete Construct or Construction		Federal Highway Administration	Lt.	Left
Const.	Construct or Construction	FIB	Florida-I Beam		
Cont.	Continuation or Continuous	F.S.	Florida Statutes		
Corr.	Corrugated	FS	Far Side		
Cov.	Cover	Ft.	Foot or Feet		
		FTP	Florida Traffic Plans		

ABBREVIATIONS

FY 2020-21 STANDARD PLANS

		F1 2020	0-21 STANDARD PLANS		
reviation .	Meaning	Abbreviation	Meaning	Abbreviation	Meaning
M m	Meter	F P.E. or PE	Professional Engineer	St. or ST.	S
m²	Meter Square	Pen.	Penetration	St. or St.	Station
Mach.	Machine	PPB	Pier Protection Barrier	Std.	Standard
MAS	Motorist Awareness System	PPP	Polypropylene pipe	Stg.	Strong
MASH	Manual for Assessing Safety Hardware (AASHTO)	Prest.	Prestressed	Stl.	Steel
Max.	Maximum	PRS	Portable Regulatory Sign	SW.	Skewed Angle
MES	Mitered End Section	psf	Pounds Per Square Foot	Swk.	Sidewalk
M.H.	Manhole or Mounting Height	PSI or psi	Pounds Per Square Inch	SYM	Symmetrical
MHW		PT PT	Point of Tangency or Pressure Treated	31W	7
	Mean High Water Middle				•
Mil or Mila	One-Thousandth Of An Inch	PTFE PVC	Polytetrafluoroethylene	T or t	Thickness, Tangent Distance or Tim
Mil or Mils			Polyvinyl Chloride	Tan	Tangent
Min.	Minimum or Minute	(5	T&G	Tongue and Groove
Misc.	Miscellaneous	Q	Flow Volume	TCP	Traffic Control Plan(s)
MLW	Mean Low Water	Qty.	Quantity	TCZ	Traffic Control Zone
mm	Millimeter	F	R	Temp.	Temperature or Temporary
Mod.	Modification	R or Rad.	Radius	Theo.	Theoretical
MOT	Maintenance Of Traffic	Rt.	Right	THW or THWN	Insulation (Flame Retardant, Moistu
MPH or mph	Miles Per Hour	R/W	Right Of Way		And Heat Resistant Thermoplastic)
MUTCD	Manual On Uniform Traffic Control Devices	RC	Reverse Crown	TMA	Truck/Trailer Mounted Attenuator
Λ	V	RCP	Reinforced Concrete Pipe	TN	Ton
N	Standard Penetration Number	Rd.	Road or Round	Trans.	Transition or Transverse
NA or N/A	Not Available or Not Applicable	Rdwy.	Roadway	TTC	Temporary Traffic Control
NC	Normal Crown	Rect.	Reticuline or Rectangular	TVSS	Transient Voltage Surge Suppressi
NCHRP	National Cooperative Highway Research Program	Ref.	Reference	TX	Transmit
NDCBU	Neighborhood Delivery And Collection Box Unit	Reinf.	Reinforced or Reinforcement	Тур.	Typical
NEMA	National Electrical Manufacturers Association	Req. or Reqd.	Required		U
NHW	Normal High Water	RGS	Rigid Galvanized Steel	UL	Underwriters Laboratories
No.	Number	RPM	Raised Pavement Markers	UPS	Uninterruptible Power Supply
Nom.	Nominal	R/R or RR	Railroad	USPS	United States Postal Service
NPS	Nominal Pipe Size	RSDU	Radar Speed Display Unit	Util.	Utilities
NPT	National Pipe Thread	RU	Rack Unit	UV	Ultraviolet
NS or N.S.	Near Side	RX	Receive		V
NS	Non-Structural	5	5	Veh.	Vehicle
NTS	Not To Scale	S or s	Speed, Spacing or Second	Vert.	Vertical
d)	Sch.	Schedule	VPD or Vpd.	Vehicles Per Day
0.C.	On Center	SHBR	Special Height Bicycle Railing	•	N
0 to 0 or 0.0.	Out to Out	Shldr.	Shoulder	W	 Width or Wide
0.B.G.	Optional Base Group	SHW	Seasonal High Water	WT	Weight
0D or 0.D.	Outside Diameter	SIP	Stay In Place	WWM	Welded Wire Mesh
0z.	Ounce	SP	Superpave	WWR	Welded Wire Reinforcing
F	D	Spa., Spcg. or Sp.	Space(ing)(s)		Y
P Pavt.	Pavement	Зра., Зрсу. от Зр. Spec.	Specification	Yd.	Yard
Pavi. PBR		•			
	Pedestrian/Bicycle Railing	sq Et SE of or SE	Square Square	Yr.	Year
PC	Point Of Curvature	Sq. Ft., SF, sf or S.F.	Square Foot		
PCC	Plain Cement Concrete	sq. in.	Square Inch		
pcf	Pounds per Cubic Foot	Sq. Yd., SY or S.Y.	Square Yard		
PCMS	Portable Changeable Message Sign	SR	State Road		
		SS	Stainless Steel		

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470	460-470	Traffic Railing - (Thrie-Beam Retrofit) General Note & Details	546	Deleted*	Sight Distance at Intersections [*Content moved to the FDM]
471	460-471	Traffic Railing – (Thrie-Beam Retrofit) Narrow Curb	560	830-T01	Railroad Crossings
472	460-472	Traffic Railing - (Thrie-Beam Retrofit) Wide Strong Curb Type 1	Traffic Conti	rol Through Wor	k Zones
473	460-473	Traffic Railing - (Thrie-Beam Retrofit) Wide Strong Curb Type 2	600	Deleted	General Information for Traffic Control Through Work Zones, New Index 102-000
474	460-474	Traffic Railing – (Thrie-Beam Retrofit) Intermediate Curb	601	Deleted	Two-Lane, Two-Way, Work Outside Shoulder, New Index 102-005
475	460-475	Traffic Railing – (Thrie-Beam Retrofit) Wide Curb Type 1	602	Deleted	Two-Lane, Two-Way, Work On Shoulder, New Index 102-010
476	460-476	Traffic Railing - (Thrie-Beam Retrofit) Wide Curb Type 2	603	Deleted	Two-Lane, Two-Way, Work Within The Travel Way, New Index 102-025
477	460-477	Thrie-Beam Panel Retrofit (Concrete Handrail)	604	Deleted	Two-Lane, Two-Way, Work in Intersection - See MUTCD.
N/A	460-490	Traffic Railing – (Rectangular Tube Retrofit)	605	Deleted	Two-Lane, Two-Way, Work Near Intersection - See MUTCD.
480	521-480	Traffic Railing - (Vertical Face Retrofit) General Notes & Details	606	Deleted	Two-Lane, Two-Way, Work Within the Travel Way - Signal Control, New Index 120-30 a 102-035
481	521-481	Traffic Railing – (Vertical Face Retrofit) Narrow Curb	607	Deleted	Two-Lane, Two-Way, Mobile Operation, Work On Shoulder and Work Within the Travel Way, New Index 102-015
482	521-482	Traffic Railing – (Vertical Face Retrofit) Wide Curb	608	Deleted	Two-Lane, Two-Way, Temporary Diversion Connection, New Index 102-040
483	521-483	Traffic Railing – (Vertical Face Retrofit) Intermediate Curb	611	Deleted	Multilane, Work Outside Shoulder, New Index 102-005
484	521-484	Traffic Railing – (Vertical Face Retrofit) Spread Footing Approach	612	Deleted	Multilane, Work on Shoulder, New Index 102-010

Design Standards Index	Standard Plans Index	Index Title	Design Standards Index	Standard Plans Index	Index Title
Traffic Contr	ol Through Wor	k Zones (Cont.)	Fencing and I	Pedestrian Rail	lings (Cont.)
613	Deleted	Multilane, Work Within Travel Way-Median or Outside Lane, New Index 102-045	821	515-021	Bridge Aluminum Pedestrian/Bicycle Bullet Railing for Traffic Railing
614	Deleted	Multilane, Work Within Travel Way-Center Lane - See Index 102-050 or MUTCD TA-38 for interior lane closures. See Index 102-055 for lane shifts.	822	515-022	Bridge Aluminum Pedestrian/Bicycle Bullet Railing Details
615	Deleted	Multilane, Work in Intersection - See MUTCD.	825	521-825	42" Concrete Pedestrian/Bicycle Railing
616	Deleted	Multilane, Work Near Intersection-Median or Outside Lane -See MUTCD.	851	515-051	Bridge Pedestrian/Bicycle Railing (Steel)
617	Deleted	Multilane, Work In Intersection - Center Lane -See MUTCD	852	515-052	Steel Pedestrian/Bicycle Railing
618	Deleted	Multilane, Work In Intersection - Two Lanes Closed-45mph or Less -See MUTCD.	861	515-061	Bridge Pedestrian/Bicycle Railing (Aluminum)
619	Deleted	Multilane, Mobile Operations Work on Shoulder, Work Within Travel Way, New Index 102- 015	862	515-062	Aluminum Pedestrian/Bicycle Railing
620	Deleted	Multilane, Divided, Temporary Diversion Connection, New Index 102-060	870	515-070	Aluminum Pipe Guiderail
621	Deleted	Multilane Undivided, Temporary Diversion Connection, New Index 102-060	880	515-080	Steel Pipe Guiderail
622	Deleted	Multilane, Work Near Intersection - Temporary Diversion Connection 35mph or Less - See MUTCD or Index 102-060.	Noise And Pe	rimeter Wall S	ystems
623	Deleted	Multilane, Work Within the Travel Way Double Lane Closure, New Index 102-050	5200	534-200	Precast Noise Walls
625	Deleted	Temporary Road Closure - 5 Minutes or Less, New Index 102-020	5210	521-510	Traffic Railing/Noise Wall (8'-0")
628	Deleted	Two Way Left Turn Lane Closure	5211	521-511	Traffic Railing/Noise Wall (14'-0")
630	Deleted	Crossover for Paving Train Operations, Rural	5212	521-512	Traffic Railing/Noise Wall (8'-0") Junction Slab
631	Deleted	Temporary Crossover	5213	521-513	Traffic Railing/Noise Wall T-Shaped Spread Footing
640	Deleted	Converting Two-Lanes to Four-Lanes Divided, Rural	5214	521-514	Traffic Railing/Noise Wall L-Shaped Spread Footing
641	Deleted	Converting Two-Lanes to Four-Lanes Divided, Urban	5215	521-515	Traffic Railing/Noise Wall Trench Footing
642	Deleted	Transitions for Temporary Concrete Barrier Wall on Freeway Facilities	5250	534-250	Perimeter Walls
650	Deleted	Two-Lane Two-Way, Rural Structure Replacement	Wall Systems	-	
651	Deleted	Multilane Divided, Maintenance and Construction, New Index 102-060	6010	400-010	C-I-P Cantilever Retaining Wall
655	Deleted	Traffic Pacing, New Index 102-070	6011	400-011	Gravity Wall
660	Deleted	Pedestrian Control for Closure of Sidewalks, New Index 102-075	6020	548-020	Permanent MSE Retaining Wall Systems
665	Deleted	Limited Access, Temporary Opening, New Index 102-065.	6030	548-030	Temporary MSE Retaining Wall Systems
667	Deleted	Toll Plaza, Traffic Control Standards	6040	455-400	Precast Concrete Sheet Pile Wall
670	Deleted	Motorist Awareness System - See Index 102-000 for MAS detail.	6100	521-600	MSE Wall Coping (Precast or C-I-P)
Fencing and I	Pedestrian Rail	ings	6110	521-610	Wall Coping With Traffic Railing/Junction Slab
800	550-004	Fence Location	6120	521-620	Wall Coping With Traffic Railing/Raised Sidewalk
801	550-001	Fence - Type A	6130	521-630	Wall Coping/Parapet with C-I-P Sidewalk
802	550-002	Fence - Type B	6200	521-650	Coping Mounted Light Pole Pedestal
803	550-003	Cantilever Slide Gate - Type B Fence	6201	521-640	Junction Slab at Drainage Inlet Openings
810	550-010	Bridge Fencing (Vertical)	Signing and M	<u>Marking</u>	
811	550-011	Bridge Fencing (Curved Top)	11200	700-020	Multi-Column Ground Sign
812	550-012	Bridge Fencing (Enclosed)	11300	700-030	Steel Overhead Sign Structures
820	521-820	27" Concrete Parapet with Pedestrian/Bicycle Bullet Railing	11310	700-040	Cantilever Sign Structure
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Design Standards Index	Standard Plans Index	Index Title	Design Standards Index	Standard Plans Index	Index Title
	Marking (Cont.)		Traffic Signa	al and Equipmen	nt (Cont.)
11320	700-041	Span Sign Structure	17733	634-002	Aerial Interconnect
11860	700-010	Single Column Ground Signs	17736	639-002	Electrical Power Service
11861	700-011	Single Column Cantilever Ground Mounted Sign	17743	649-030	Standard Mast Arm Assemblies
11862	700-120	Roadside Flashing Beacon Assembly	17745	649-031	Mast Arm Assemblies
11862	654-001	Rectangular Rapid Flashing Beacon Assembly	17748	700-050	Free-Swinging Internally-Illuminated Street Sign Assemblies
11870	700-012	Single Post Bridge Mounted Sign Support	17764	653-001	Pedestrian Control Signal Installation Details
11871	700-013	Single Post Median Barrier Mounted Sign Support	17781	660-001	Vehicle Loop Installation Details
13417	700-110	Mounting Exit Number Panels To Highway Signs	17784	665-001	Pedestrian Detector Assembly Installation Details
17302	700-101	Typical Sections For Placement of Single & Multi-Column Signs	17841	676-010	Cabinet Installation Details
17328	700-108	Typical Signing for Truck Weigh & Inspection Stations	17870	671-001	Standard Signal Operating Plans
17344	Deleted*	School Signs & Markings [*Content moved to Speed Zone Manual]	17881	509-100	Advance Warning For R/R Crossing
17345	711-003	Interchange Markings	17882	509-070	Railroad Grade Crossing Traffic Control Devices
17346	711-001	Pavement Markings	17890	508-T01	Traffic Control Devices For Movable Span Bridge Signals
17347	711-002	Bicycle Markings	Planning		
17349	700-109	Traffic Controls For Street Terminations	17900	695-001	Traffic Monitoring Site
17350	700-104	Signing For Motorist Services	Intelligent Tr	ansportation S	ystems (ITS)
17351	700-105	Welcome Center Signing	18100	Deleted	CCTV Pole Placement
17352	706-001	Typical Placement Of Reflective Pavement Markers	18101	Deleted*	Typical CCTV Site [*Combined with CCTV Indexes]
17354	Deleted*	Tourist Oriented Directional Signs [*Content moved to the FDM]	18102	Deleted*	Grounding And Lightning Protection [*Combined with CCTV and DMS Indexes]
17355	700-102	Special Sign Details	18104	Deleted	Typical CCTV Cabinet Equipment Layout
17356	659-010	Span Wire Mounted Sign Details	18105	Deleted	CCTV Block Diagram
17357	700-107	Bridge Weight Restrictions	18107	Deleted*	Ground Mounted CCTV Cabinet [*Combined with CCTV Indexes]
17359	700-106	Rural Narrow Bridge Treatment	18108	Deleted*	Pole Mounted CCTV Cabinet [*Combined with CCTV Indexes]
Roadway Ligh	nting		18110	659-020	Camera Mounting Details
17500	715-001	Conventional Lighting	18111	649-020	Steel CCTV Pole
17502	715-010	High Mast Lighting	18113	641-020	Concrete CCTV Pole
17504	639-001	Service Point Details	18300	700-090	Dynamic Message Sign Walk-In
17505	700-031	External Lighting For Signs	N/A	700-091	Catwalk Details
17515	715-002	Standard Aluminum Lighting	Prestressed	Concrete Beam	S
Traffic Signa	al and Equipmen	nt	20010	450-010	Typical Florida-I Beam Details and Notes
17700	635-001	Pull & Splice Box	20036	450-036	Florida-I 36 Beam - Standard Details
17721	630-001	Conduit Installation Details	20045	450-045	Florida-I 45 Beam - Standard Details
17723	649-010	Steel Strain Pole	20054	450-054	Florida-I 54 Beam - Standard Details
17725	641-010	Concrete Poles	20063	450-063	Florida-I 63 Beam - Standard Details
17727	634-001	Signal Cable & Span Wire Installation Details	20072	450-072	Florida-I 72 Beam - Standard Details

Design Standards Index	Standard Plans Index	Index Title	Design Standards Index	Standard Plans Index	Index Title
Prestressed	Concrete Beams	s (Cont.)	Structures A	ccess and Light	ting
20078	450-078	Florida-I 78 Beam - Standard Details	21200	521-660	Light Pole Pedestal
20084	450-084	Florida-I 84 Beam - Standard Details	21210	630-010	Conduit Details
20096	450-096	Florida-I 96 Beam - Standard Details	21220	510-001	Navigation Light System Details (Fixed Bridges)
20120	450-120	AASHTO Type II Beam	21240	715-240	Maintenance Lighting For Box Girders
20199	450-199	Build-Up & Deflection Data For Prestressed I-Beams	21250	460-250	Access Hatch Assembly For Steel Box Sections
20210	450-210	Typical Florida-U Beam Details and Notes	21251	460-251	Access Hatch Assembly For Concrete Box Sections
20248	450-248	Florida-U 48 Beam - Standard Details	21252	460-252	Access Door Assembly For Concrete Box Sections
20254	450-254	Florida-U 54 Beam - Standard Details	Standard Ba	r Bending Deta	ils
20263	450-263	Florida-U 63 Beam - Standard Details	21300	415-001	Standard Bar Bending Details
20272	450-272	Florida-U 72 Beam - Standard Details	Temporary D	etour Bridges	
20299	450-299	Build-Up and Deflection Data For Florida-U Beams	21600	102-200	Temporary Detour Bridge General Notes and Details
Bridge Beari	ings		21610	102-210	Temporary Detour Bridge Details - Timber Pile Foundations
20502	450-502	Beveled Bearing Plate Details - Prestressed Florida-U Beams	21620	102-220	Temporary Detour Bridge Details - Steel H Pile Foundations
20510	400-510	Composite Elastomeric Bearing Pads-Prestressed Florida-I & AASHTO Type II Beams	21630	102-230	Temporary Detour Bridge Details - Steel Pipe Pile Foundations
20511	450-511	Bearing Plates (Type I) - Prestressed Florida-I & AASHTO Type II Beams	21640	102-240	Temporary Detour Bridge Thrie-Beam Guardrail
20512	450-512	Bearing Plates (Type 2) - Prestressed Florida-I & AASHTO Type II Beams	Post-Tension	ing	
Square and I	Round Concrete	Piles (With Carbon Steel)	21801	462-001	Post-Tensioning Vertical Profile
20600	455-001	Notes and Details For Square Prestressed Concrete Piles	21802	462-002	Post-Tensioning Anchorage Protection
20601	455-002	Square Prestressed Concrete Pile Splices	21803	462-003	Post-Tensioning Anchorage and Grouting Details
20602	455-003	EDC Instrumentation For Square Prestressed Concrete Piles	Fender Syste	em Details	
20612	455-012	12" Square Prestressed Concrete Pile	21930	471-030	Fender System - Prestressed Concrete Piles
20614	455-014	14" Square Prestressed Concrete Pile	Wall Systems	(Corrosion Re	sistant)_
20618	455-018	18" Square Prestressed Concrete Pile	22440	455-440	Precast Concrete CFRP/GFRP & HSSS/GFRP Sheet Pile Wall
20620	455-020	20" Square Prestressed Concrete Pile	Square and I	Round Concrete	Piles (Corrosion Resistant)
20624	455-024	24" Square Prestressed Concrete Pile	22600	455-101	Notes and Details for Square CFRP & SS Prestressed Concrete Piles
20630	455-030	30" Square Prestressed Concrete Pile	22601	455-102	Square CFRP and SS Prestressed Concrete Pile Splices
20631	455-031	High Moment Capacity 30" Square Prestressed Concrete Pile	22612	455-112	12" Square CFRP and SS Prestressed Concrete Pile
20654	455-054	54" Precast/Post- Tensioned Concrete Cylinder Pile	22614	455-114	14" Square CFRP and SS Prestressed Concrete Pile
20660	455-060	60" Prestressed Concrete Cylinder Pile	22618	455-118	18" Square CFRP and SS Prestressed Concrete Pile
Approach Sla	abs		22624	455-124	24" Square CFRP and SS Prestressed Concrete Pile
20900	400-090	Approach Slabs (Flexible Pavement Approaches)	22630	455-130	30" Square CFRP and SS Prestressed Concrete Pile
20910	400-091	Approach Slabs (Rigid Pavement Approaches)	22654	455-154	54" Square CFRP and SS Prestressed Concrete Pile
Bridge Expai	nsion Joints		22660	455-160	60" Square CFRP and SS Prestressed Concrete Pile
21100	458-100	Strip Seal Expansion Joint			
21110	458-110	Poured Joint With Backer Rod Expansion Joint System			<u>.</u>

Standard Plans Index	Description
102-000	NEW INDEX - Previously Index 102-600; Reorganized Index and Series; Added "Temporary Traffic Control Tables"; Added "Temporary Raised Rumble Strips" detail; Added "Motorist Awareness System" detail.
102-005	NEW INDEX - Work Beyond the Shoulder.
102-010	NEW INDEX - Work on the Shoulder.
102-015	NEW INDEX - Mobile Operations.
102-020	NEW INDEX - Temporary Roadway Closure.
102-025	NEW INDEX – Two-Lane Roadway, Lane Closure Using Flagger.
102-030	NEW INDEX - Two-Lane Roadway, Lane Closure Using Temporary Traffic Signals.
102-035	NEW INDEX - Haul Road Crossing.
102-040	NEW INDEX - Two-Lane Roadway, Temporary Diversion.
102-045	NEW INDEX - Multilane Roadway, Single Lane Closure.
102-050	NEW INDEX - Multilane Roadway, Multiple Lane Closures.
102-055	NEW INDEX - Multilane Roadway, Lane Closure with Lane Shift.
102-060	NEW INDEX - Multilane Roadway, Temporary Diversion.
102-065	NEW INDEX - Limited Access Temporary Opening.
102-070	NEW INDEX - Traffic Pacing.
102-075	NEW INDEX - Work on the Sidewalk.
102-110	Sheet 15: Updated CONCRETE Note "B" to reference Specification 346-10.
102-600	Deleted - Moved to New Index 102-000.
102-601	Deleted - Moved to New Index 102-005.
102-602	Deleted - Moved to New Index 102-010.
102-603	Deleted - Moved to New Index 102-025.
102-604	Deleted Index. See MUTCD for intersection TTC.
102-605	Deleted Index. See MUTCD for intersection TTC.
102-606	Deleted - Moved to New Index 120-030 and 102-035.
102-607	Deleted - Moved to New Index 102-015.
102-608	Deleted - Moved to New Index 102-040.
102-611	Deleted - Moved to New Index 102-005.
102-612	Deleted - Moved to New Index 102-010.
102-613	Deleted - Moved to New Index 102-045.
102-614	See Index 102-050 or MUTCD TA-38 for interior lane closures. See Index 102-055 for lane shifts.
102-615	Deleted Index. See MUTCD for intersection TTC.
102-616	Deleted Index. See MUTCD for intersection TTC.
102-617	Deleted Index. See MUTCD for intersection TTC.
102-618	Deleted Index. See MUTCD for intersection TTC.
102-619	Deleted - Moved to New Index 102-015.
102-620	Deleted - Moved to New Index 102-060.
102-621	Deleted - Moved to New Index 102-060.
102-622	Deleted Index. See MUTCD TA-32 or Index 102-060.

Standard Plans Index	Description
102-623	Deleted - Moved to New Index 102-050.
102-625	Deleted - Moved to New Index 102-020.
102-628	Deleted Index.
102-630	Deleted Index.
102-631	Deleted Index.
102-640	Deleted Index.
102-641	Deleted Index.
102-642	Deleted Index.
102-650	Deleted Index.
102-651	Deleted - Moved to New Index 102-060.
102-655	Deleted - Moved to New Index 102-070.
102-660	Deleted - Moved to New Index 102-075.
102-665	Deleted - Moved to New Index 102-065.
102-667	Deleted Index.
102-670	Deleted Index; See Index 102-000 for MAS detail.
160-001	Changed Index Title to: "Median Stabilizing Details".
350-001	Sheet 1: Clarified Note 8.
370-001	Changed Index Title to: "Bridge Approach Expansion Joint Concrete Pavement With Special Select Soil Base"; Deleted Design Notes; Updated General Notes.
425-010	Sheet 1: Added a 4'-0" diameter option in ALTERNATE B SECTION B-B and ROUND RISER OPENING detail.
425-031	All Sheets: Changed Index Title to "Adjacent Barrier Inlet". Sheet 1: Changed General Note 1 to include median barriers with usage; Updated section detail labels to include median barriers.
430-001	Reorganized Index; Added additional Sheets. Sheet 1: Limits of Variable Front Slopes at Drainage Structures. Sheet 2: Round and Elliptical Concrete Pipe Joints. Sheet 3: Filter Fabric Jacket, Concrete Jacket, and Pipe Plug. Sheet 4: Concrete Collars. Sheet 5: Pipe End Guard. Sheet 6: Retaining Wall Concrete Gutter and Drains.
430-010	Reorganized Index; Added additional Sheets; Moved Sodding quantities to Index 570-001. Sheet 1: General Notes and Overview; Front Slope Transition at Endwall; Moved General Note 1 to the SPI; Moved specification and payment information to Specifications; Added General Note on quantities for estimating purposes only. Sheet 2: Dimensional and Reinforcing Details. Sheet 3: Type 1 and Type 2 Grate Details.
430-011	Reorganized Index; Added additional Sheets. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications Added General Note on quantities for estimating purposes only. Sheet 2: Endwalls for 1:2 Slopes with Baffles; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars. Sheet 3: Endwalls for 1:2 Slopes Without Baffles and Bar Bending Diagram; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars. Sheet 4: Endwalls with and Without Baffles for 1:3, 1:4, and 1:6 Slopes. Sheet 5: Steel Grate Option; Steel Grating Use Criteria Moved to SPI.

Standard Plans Index	Description
430-012	Reorganized Index; Added additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Added General Note on quantities for estimating purposes only. Sheet 2: Dimension Details. Sheet 3: Reinforcing Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars.
430-020	Reorganized Index; Added additional Sheet. Sheet 1: General Notes and Overview; Moved payment information to Specifications; Added General Note on quantities for estimating purposes only. Sheet 2: Straight Flare and Optional Shape.
430-021	Reorganized Index; Moved payment information to Specifications. Sheet 1: General Notes and Overview and Slope And Ditch Transitions. Design Notes to the Drainage Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Combined details for Round and Elliptical Concrete Pipe; Added Section A-A "Pipe/Slab Fillet"; Added DETAIL "A". Sheet 3: Combined Tables of Quantities for Round and Elliptical Concrete Pipe. Sheet 4: Combined details for Arched and Round Corrugated Metal Pipe. Sheet 5: Combined Tables of Quantities for Arched and Round Corrugated Metal Pipe. Sheet 6: Moved Connection and Anchor Details.
430-022	Reorganized Index; Moved payment information to Specifications. Sheet 1: General Notes and Overview and Ditch Transitions; Moved Design Notes to the SPI; Deleted information already in the Drainage Design Guide; Added General Note on Saddle Slope; Added General Note on quantities for estimating purposes only. Sheet 2: Combined details for Round and Elliptical Concrete Pipe; Added Section A-A "Pipe/Slab Fillet"; Added DETAIL "A". Sheet 3: Combined Tables of Quantities for Round, Elliptical Concrete Pipe, and Permissible Pavement Modification detail. Sheet 4: Combined details for Arched and Round Corrugated Metal Pipe. Sheet 5: Combined Tables of Quantities for Arched and Round Corrugated Metal Pipe. Sheet 6: Moved Connection and Anchor Details. Sheet 7: Moved Fastener and Grate Details.
430-030	Reorganized Index; Added additional Sheet. Sheet 1: General Notes and overall view; Moved specification and payment information to Specifications; Added General Note on quantities for estimating purposes only. Sheet 2: Concrete Endwall Details; Added Sta,/Offset Location. Sheet 3: Combined Quantities Tables; Elliptical Concrete and Elliptical/ Arched Corrugated Metal Pipe tables. Sheet 4: Moved Spacing for Multiple Pipes Details.
430-031	Reorganized Index; Added additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Single 60" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars. Sheet 3: Double 60" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars.
430-032	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Single 66" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars. Sheet 3: Double 66" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars.

Standard Plans Index	Description
430-033	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Single 72" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars. Sheet 3: Double 72" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars.
430-034	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Single 84" Endwall Details; Updated bar naming conventions to reflect Horizontal, Vertical, and Bent Bars.
430-040	Reorganized Index; Renamed Index to: "Winged Concrete Endwalls"; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Endwall With U-Type Wings and Endwall With 45 Degree Wings.
430-090	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved specification and payment information to Specifications; Deleted design information covered in the Design Manual; Added General Note on quantities for estimating purposes only. Sheet 2: Endwalls for 1:4 and 1:6 Slopes; Split detail into Dimensional and Reinforcing Details. Sheet 3: Steel Grate Details.
436-001	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved Design Notes to the Drainage Manual, Drainage Design Guide, and the FDOT Design Manual. Sheet 2: Type I - Nonremovable Grate. Sheet 3: Type II - Removable Grate.
440-001	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Moved Design Notes to the SPI. Sheet 2: Type I, II, and III Underdrains. Sheet 3: Type Va, Vb, Underdrains and Cleanout.
440-002	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Updated Notes. Sheet 2: Typical Inspection Box Installation. Sheet 3: Typical Urban, Slope, and Adjustment Installations.
443-001	Reorganized Index; Added Additional Sheet. Sheet 1: General Notes and Overview; Updated Notes; Removed Design Notes and moved to the SPI and Drainage Manual. Sheet 2: French Drain System. Sheet 3: Concrete Slotted Pipe Options.
443-002	Reorganized Index; Added additional Sheet. Sheet 1: General Notes and Overview; Moved Design Notes to SPI and Drainage Manual. Sheet 2: Type I Skimmers. Sheet 3: Type II Skimmers.
446-001	Reorganized Index; Moved payment information to Specifications; Deleted Treated Permeable Base Subdrainage. Sheet 1: General Notes and Overview. Sheet 2: Sudrainage and Outlet. Sheet 3: New Construction. Sheet 4: Rehabilitation.

Standard Plans Index	Description				
450-010	Sheet 2: Removed INSERT DETAIL.				
450-036	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-045	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-054	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-063	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-072	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-084	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-096	Sheet 1: Deleted Intermediate Diaphragm Inserts.				
450-120	Sheet 2: Deleted INSERT DETAIL. Sheet 3: Deleted Intermediate Diaphragm Inserts.				
455-440	Sheet 2: Changed bend diameter of GFRP stirrups; Added new Note 5 and renumbered Notes.				
458-100	Changed Elastomeric Seal to Strip Seal throughout Index.				
460-470	Sheet 1: Changed Barrier Delineator note.				
515-052	Sheet 8: Changed embedment depths and anchor lengths for Case I and Case IIb.				
515-062	Sheet 9: Changed embedment depths and anchor lengths for Case I and Case IIb.				
521-001	Sheets 15 & 16: Changed "Shoulder Barrier Inlet" to "Adjacent Barrier Inlet" callout.				
521-002	Sheet 1: Updated Note 4 to refer to Index 425-031 for "Adjacent Barrier Inlets" (number correction and Index name update).				
521-404	Sheet 1: Changed Payment Note.				
521-405	Sheet 1: Changed Payment Note.				
521-427	Sheets 1 - 4: Renumbered. Sheet 5: (NEW SHEET) Drainage Slot Details.				
521-428	Sheet 1: Added reference to drainage slot detail. Sheet 3: Added transition from 42" on Bridge to 36" or 38" traffic railing on approaches.				
521-480	Sheet 1: Removed Barrier Delineator Spacing table; Changed Barrier Delineator note to refer to specification 705 instead of table.				
521-510	Sheet 1: Changed 10' maximum spacing for 1/2" V-Groove in consideration of 12' precast sections.				
521-600	Sheet 1: Added organic felt bond breaker on surfaces of wall between C-I-P coping to prevent cracking of the coping and wall. Sheet 2: Added bond breakers between face of wall and C-I-P coping.				
521-610	Sheet 1: Corrected Note referenced in Partial Plan View for Approach Slab, Sheet 2: Clarified Note 4; Added Overbuild to Typical Section; Changed Title to End Transition Details. Sheet 3: Changed Detail "A" to Details "B"; Clarified alternate construction joint for Detail "B".				
521-620	Sheet 1: Corrected reference to Approach Slab Note in Partial Plan View; Changed maximum spacing of 3/4" expansion joints.				

Standard Plans Index	Description				
521-640	Added Note 7 and renumbered Notes; Locate Open Joints in Barrier & Coping a minimum of 5'-0 from CL of Barrier Wall Inlet.				
521-660	Sheets 1, 2 & 4: Removed notes to slope concrete pedestal surface.				
522-002	Deleted General Note 2 regarding parallel grade break; Renumbered General Notes based on the deletion of Note 2.				
524-001	Removed Sodding information from Sheet 2 and Added to new Sheet 3 in Index 570-001.				
534-200	Sheet 1: Changed Note 6.C.1.				
536-001	All Sheets: Renumbered for additional Sheets 14 and 16. Sheet 1: Added Trailing End Transition Connection to Rigid Barrier to Table of Contents; Added Sheets 14 & 16 to TOC and renumbered; Removed flared approach terminal from TOC; Note 10, allow for single-reduced post spacing for connections to existing guardrail. Sheets 2 & 3: Removed modified thrie beam from Note 8. Sheet 5: Removed modified thrie beam details and Note 5; Added single-faced to double-faced guardrail connection detail. Sheet 6: Removed modified thrie-beam section and post information from table. Sheet 7: Removed flared approach terminal; Added note 6 "clear area requirement"; Added approach terminal callout at begin/end guardrail location; Added new information to Note 5 to allow substitution for miscellaneous asphalt pavement placed upstream of post 1. Sheet 8: Renumbered Note 7 to Note 9 and deleted reference to flare; Added a new Note "Clear Area Requirement"; Added approach terminal callout at begin/end guardrail location; Added new Note 8 to allow substitution for miscellaneous asphalt pavement placed upstream of post 1. Sheet 9: Added trailing anchorage callout to being/end guardrail station location. Sheet 13: Changed default curb option shown to "Flush Shoulder Option", added TL-3 approach transition callout at begin/end GR. location. Sheet 14: (NEW SHEET): Added full TL-3 Approach Transition Connection layout for both curb continuation options; show guardrail tapers. Sheet 15: (Previously Sheet 14): Changed default curb option shown to "Flush Shoulder Option"; added TL-2 approach transition callout at begin/end GR. Location. Sheet 16: (NEW SHEET): Added full TL-2 Approach Transition Connection layout for both curb continuation options; show Guardrail tapers. Sheet 17: (Previously Sheet 17): Terminal updated from flared to parallel in Plan View; Added approach transition callout at begin/end guardrail location. Sheet 20: (Previously Sheet 17): Updated terminal from flared to parallel in Plan View, Sheet 21: (Previously Sh				
536-002	Sheet 1: Updated Note 2 to remove trailing end transition information; Reference Index 536-001 for new trailing end transition connection details on New Sheet 28. Sheet 2: Removed Detail K references in details (typo from old Standard version). Sheet 3: Changed approach terminals from flared to parallel. Sheet 27: Removed Payment Information; Updated detail title to sync with Pay Item title, "Guardrail Approach Transition Connections". Sheet 28: (NEW SHEET) Developed for Trailing End Transition Connections.				

Standard Plans Index	Description			
544-001	Sheet 1: Added callout notes to rigid barrier connection detail to include traffic railing and concrete barrier standards. Sheet 3: (NEW SHEET) Added Thrie Beam retrofit connection detail; Short guardrail extension options from crash cushion to rigid barrier.			
570-001	All Sheets: Renumbered for additional New Sheet. Sheet 3: (NEW SHEET) Added sodding information from Index 524-001.			
580-001	Updated bracing Detail, overall clarifications; Clarified that bracing is intended for plant establishment purposes only; Clarified on lumber grade; Clarified band strength.			
591-001	NEW INDEX - Previously Developmental Standard Plan D591-001.			
639-001	Corrected TYPICAL DISTRIBUTION POINT SCHEMATIC DETAIL Callout and "OFF" position location.			
649-010	Added Longitudinal Seam weld note 4H; Changed Note 4H to 4I.			
649-031	Sheet 2: Change "jam nut" to anchor nut to match spec language. Sheet 4: Clarified splice length (to match Sheet 3 splice). Sheet 5: Clarified that the luminaire arms are galvanized steel.			
695-001	Sheet 5: Changed the PVC Conduit or Non-Metallic Flexible Conduit from 1.5" to 3".			
700-010	All Sheets: Renumbered. Sheet 6: Note 5: Clarified number of wind beams required. Sheet 7: (NEW SHEET) - WIND BEAM CONNECTION FOR FLIP UP SIGN.			
700-041	Sheet 1: Added information to Note 5B and Note 5C.			
700-090	Deleted Catwalk Notes and references and added to New Index 700-091.			
700-091	NEW INDEX - Catwalk Details.			
700-101	Changed lateral offsets to more closely correspond with the MUTCD.			
700-104	Note 4: Updated terminology for sign posts to match current naming.			
700-110	Deleted 30 degree cut of Z mounting beams and added bolt diameters to drawing (See Index 700-030).			
706-001	Changed striping limits in detail.			
711-001	Sheet 1: Clarified "Notes for Pavement Message". Sheet 2: Changed "Contrast Markings with Alternating Skip Pattern" to "10'-30' Skip Line with Shadow Markings". Added "Dotted Line with Alternating Shadow Markings" with detail. Sheet 5 & 6: Deleted the 6" Yellow marking from the nose of the Traffic Separator. Sheet 8: Revised right turn lane details. Sheet 11: Revised all details and notes.			
715-002	Sheet 1: Changed Note 4C. Sheet 2: Added 20' & 22' mounting heights. Sheet 3: Changed Strut weld size in ARM ELEVATION Detail. Sheet 4: Added Pole PO.			

Standard Plans Index	Description
715-010	Sheet 3: Updated handhole ring and door dimensions to allow variation/increase in handhole size; increase distance from baseplate to bottom of handhole.
830-001	Sheet 2: Added a line indicating the curb continuing to the junction with the crossing; Remove the label about shoulder pavement in lieu of curb; Added a label for drop curb; Modified label "shoulder pavement" to "asphalt pavement" on the left half; Added "or trail" label to sidewalk on right half; Changed "shoulder pavement" on right to asphalt or concrete pavement to match adjacent surface.

REVISIONS PLACE HOLDER

Work this Index with Index 102-210, 102-220, 102-230 and 102-240.

STRUCTURAL STEEL:

Steel Plates and Rolled Sections shall be ASTM A709 Grade 36. Pipe piles shall be ASTM A252 Grade 2, Fy = 35 ksi.

BOLTS, LAG SCREWS AND THREADED BOLT STOCK:

Furnish high strength bolts in accordance with ASTM F3125 Grade A325 Type 1. Furnish Threaded Stock in accordance with ASTM A36. Furnish Lag Screws in accordance with ASTM A307. Furnish steel washers and nuts compatible with Bolts, Threaded Stock and Lag Screws.

TIMBER AND LAGGING:

Timber and Lagging shall be No. 1 Southern Yellow Pine.

BACKWALL BENT PILES:

Timber Piles:

10' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6). Ultimate Capacity greater than 18 tons. Splices are not allowed on any timber piles.

12' Minimum Embedment into compacted backfill or into soil having a blow count greater than 6 (N>6). Ultimate Capacity greater than 18 tons.

Shims admissible between backwall pile and cap. Test piles are not required for backwall piles.

EXPANSION BEARINGS:

Inspect the PTFE (Teflon) layer and stainless steel plate prior to installation. Do not use bearings that have a severely damaged or unbonded PTFE layer. Clean PTFE of all grit and grime prior to installation. Clean Stainless steel plate of all grit and grime prior to installation and finish to a smooth buffed surface.

DISTRIBUTING BEAMS:

Longitudinal stops restraining the distributing beams may be lengthened or shortened to center the distributing beam bearing on the cap beam. The longitudinal stops are to bear on the distributing beam end frame.

Install the expansion joint considering the total continuous bridge length, location of fixed bearings and ambient temperature at the time of installation, assume a 1" expansion joint opening at 70 degrees F.

STORAGE FACILITY:

Contact FDOT Statewide Aluminum Shop 2590 Camp Rd. Oviedo, Fl. 407-278-2727

For shipping weights and dimensions of Temporary Bridge elements.

SHIPPING WEIGHTS AND DIMENSIONS:

Decking Sizes:

Type	Length	Width	Weight (lbs.)
Curb	5'	6'-9"	800
Curb	1 O'	6'-9"	1420
Curb	15'	6'-9"	2200
Curb	20'	6'-9"	2800
NonCurb	5'	5'-3''	650
NonCurb	10'	5'-3''	1000
NonCurb	15'	5'-3''	1600
NonCurb	20'	5'-3"	2100

Shipping weights and dimensions of other bridge components can be referenced in "Acrow Panel Bridging, Series 300, Technical Handbook".

TRAFFIC RAILING NOTES:

See Index 536-001 for component details, geometric layouts and associated notes not fully detailed herein.

CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements of AASHTO M180, Type II (Zinc coated).

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Do not drill Temporary Bridge components to attach Guardrail. Guardrail Bolts shall be placed between Truss members as shown in Index 102-240.

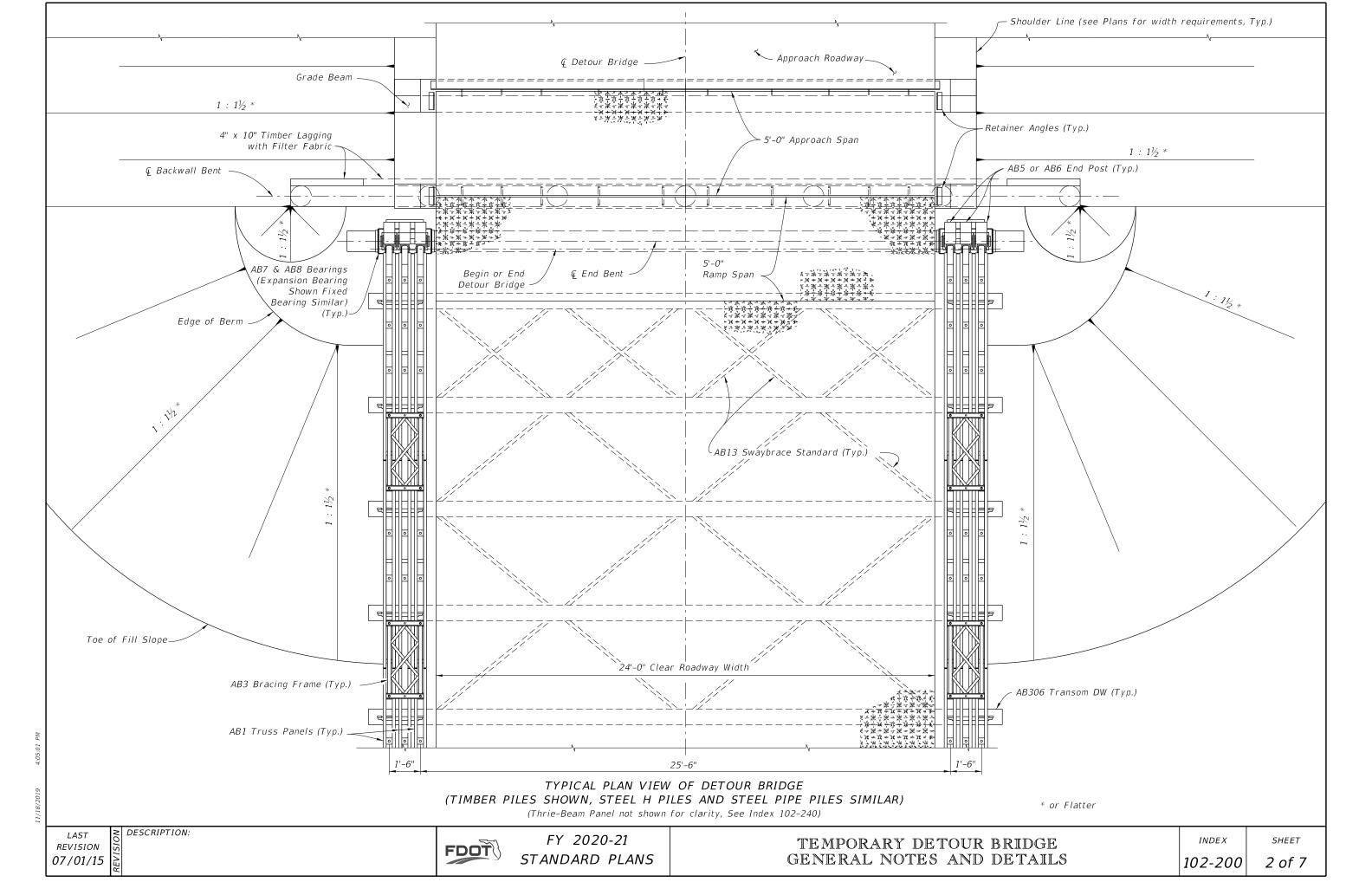
COATINGS: All Nuts, Bolts, Anchors, Washers and Backer Plates shall be hot-dip galvanized in accordance with the Specifications.

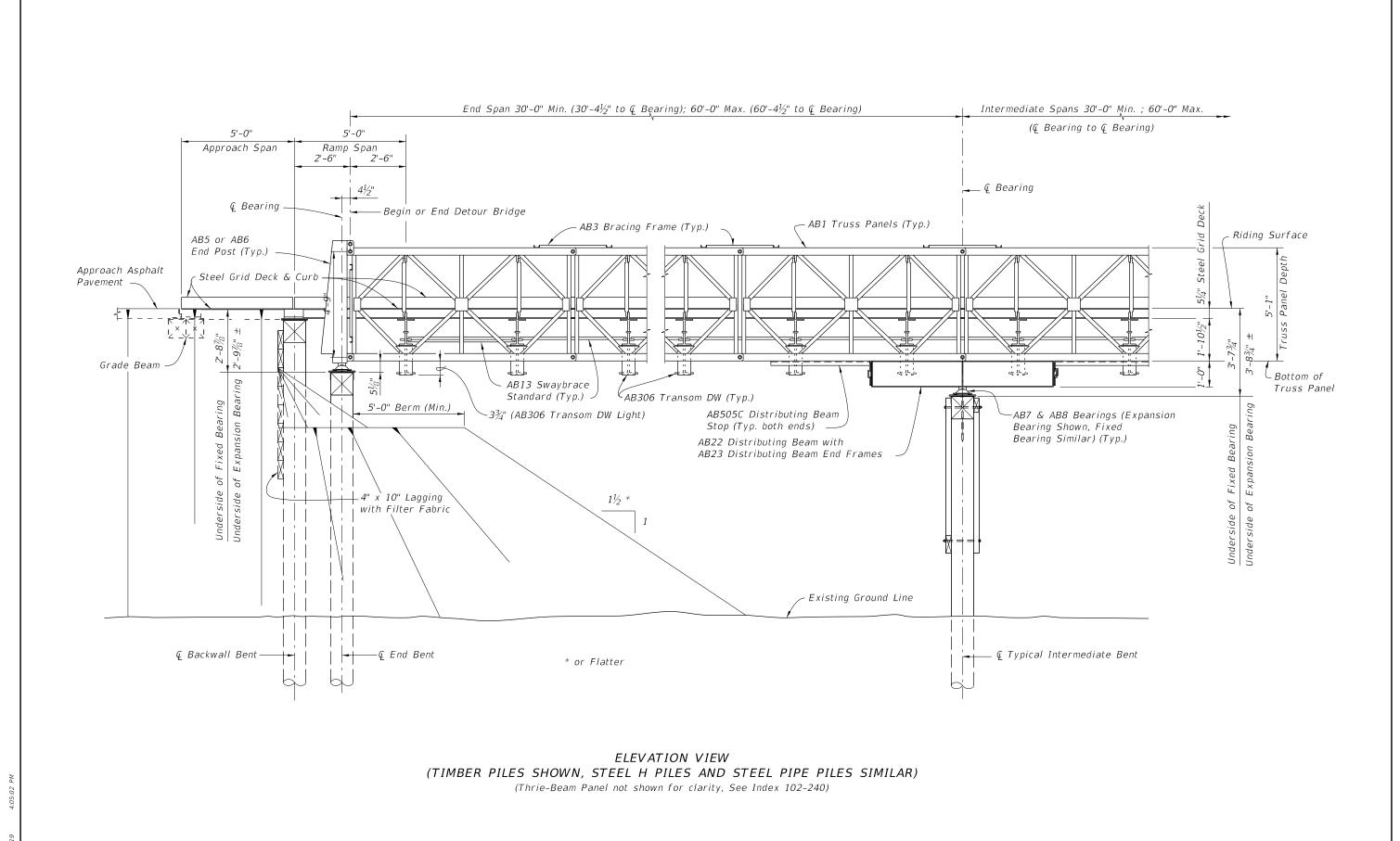
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered $(\pm 1/4")$.

PAYMENT:

Temporary Detour Bridge is to be paid for under Contract Unit Price for Special Detour. If a temporary bridge system other than that shown herein is used, the Contractor is responsible for renting or purchasing their own system. Payment for Temporary Guardrail work and Transition Block will be made under Pay Item Temporary Guardrail, LF.

Furnish and install Bridge Thrie-Beam Panels and all associated hardware as shown. Payment will be made with the Temporary Detour Bridge under the Pay Item Special Detour, LS. Turn over Bridge Thrie-Beam Panels and all associated hardware to the Department with the Detour Bridge components per Specifications Section 102-6.

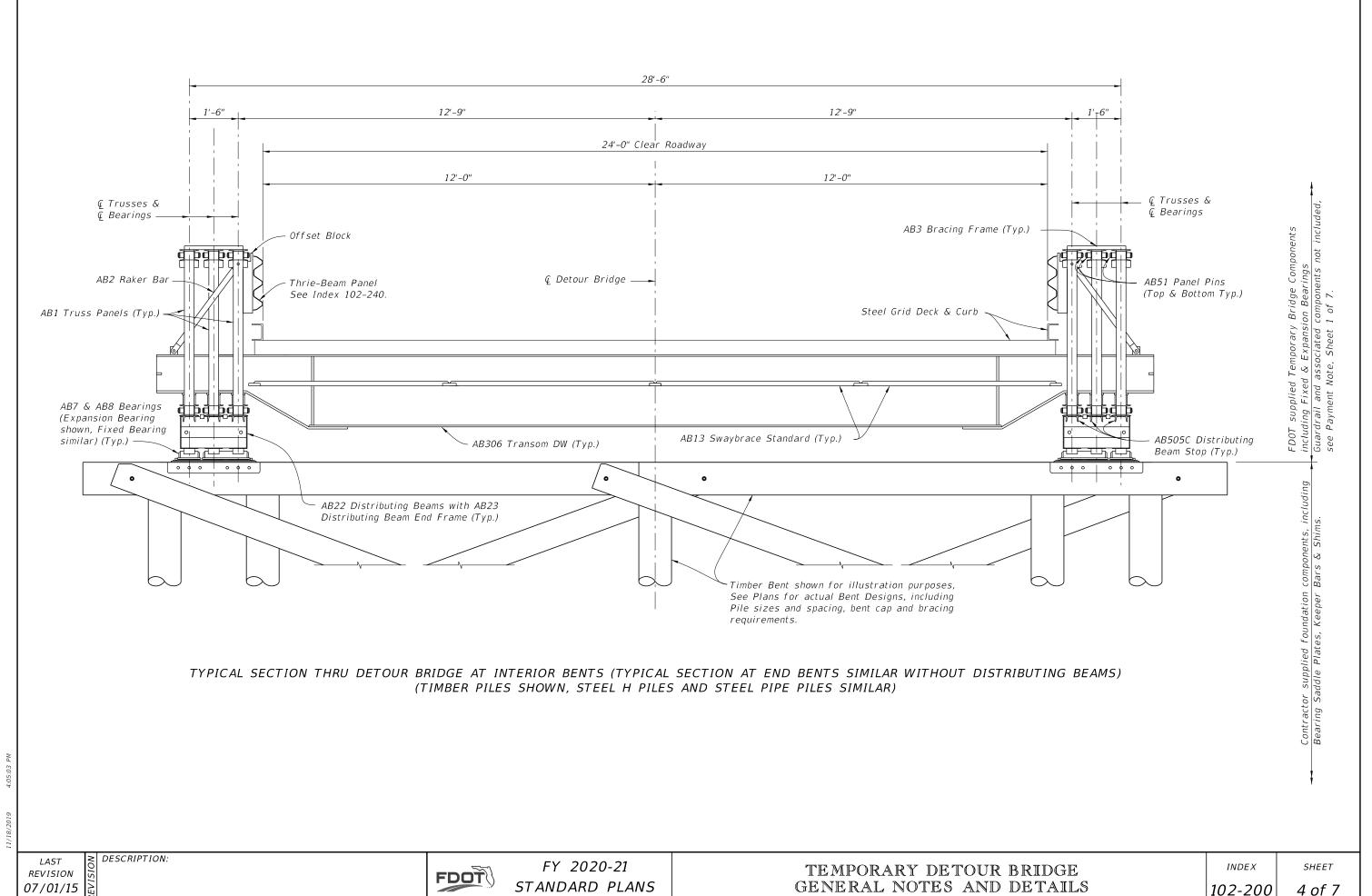


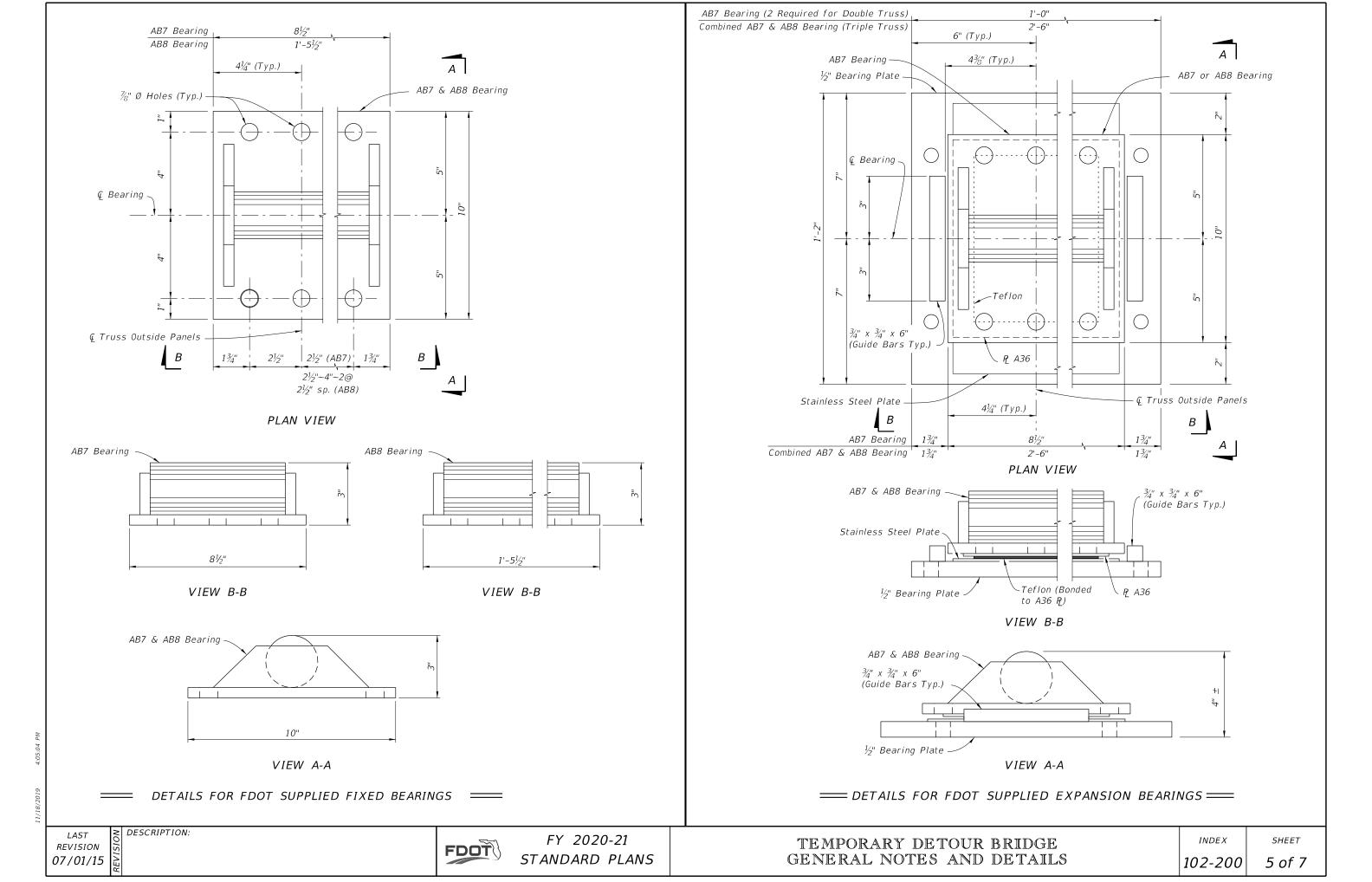


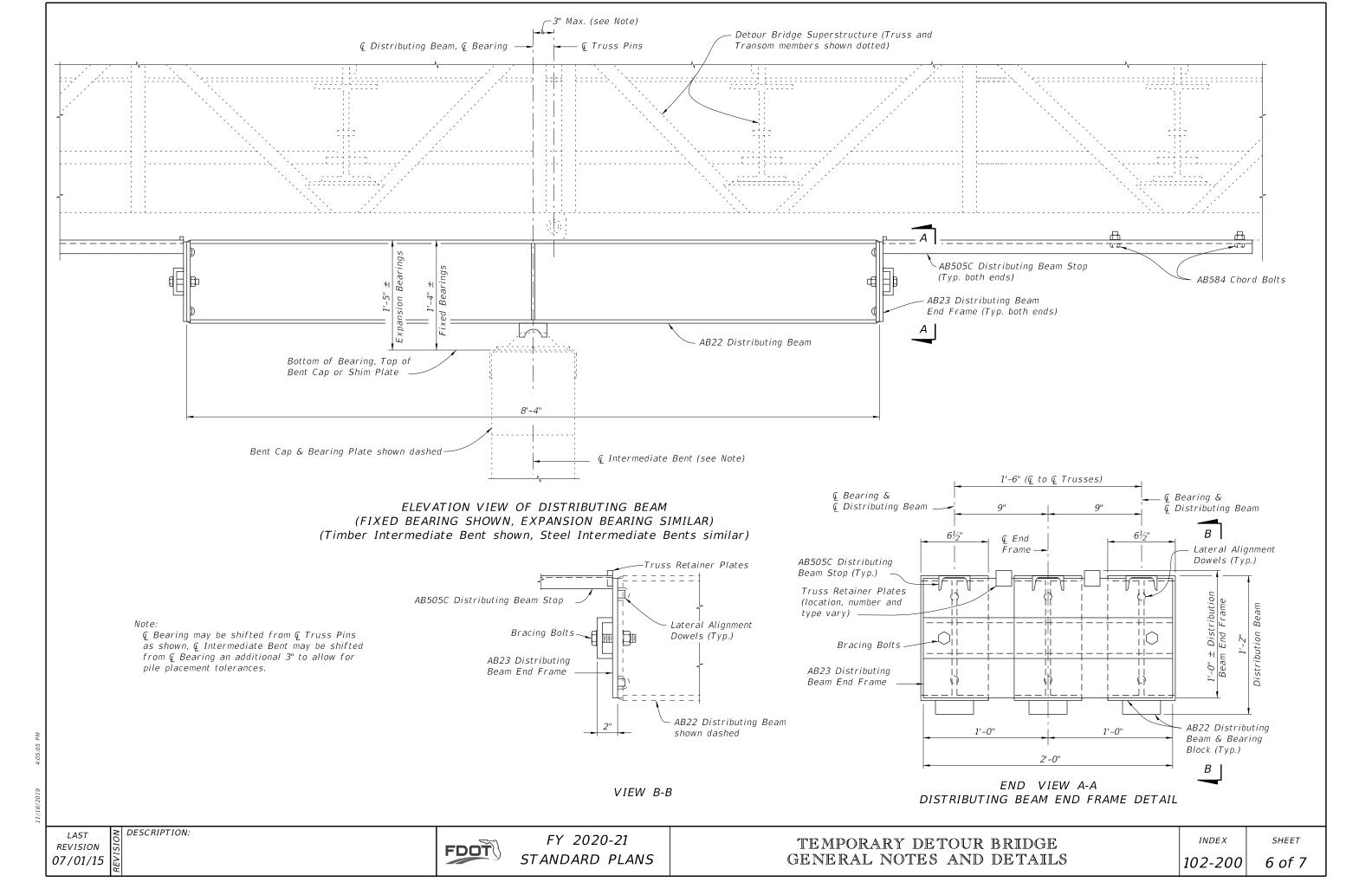
REVISION 07/01/15

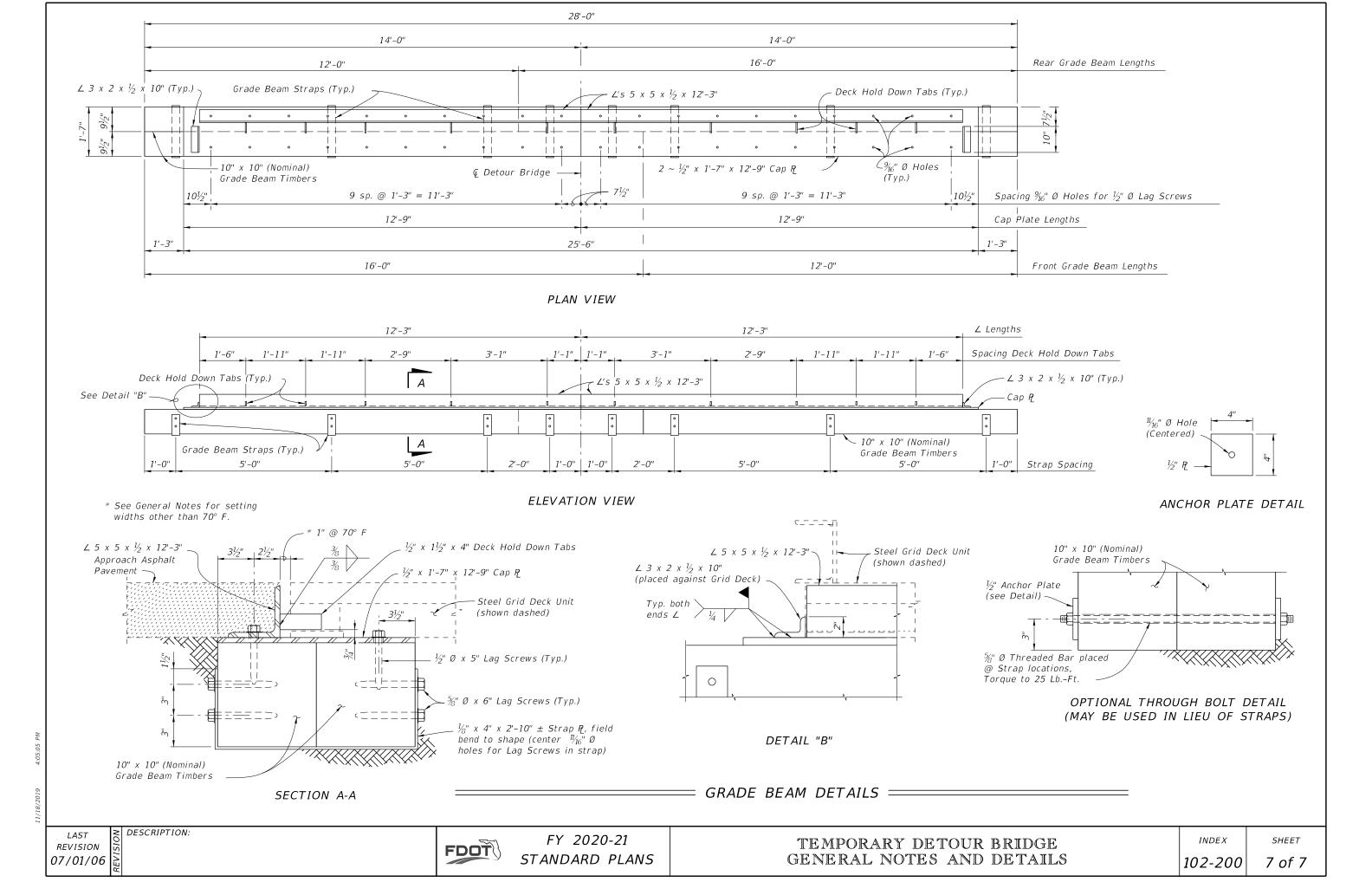
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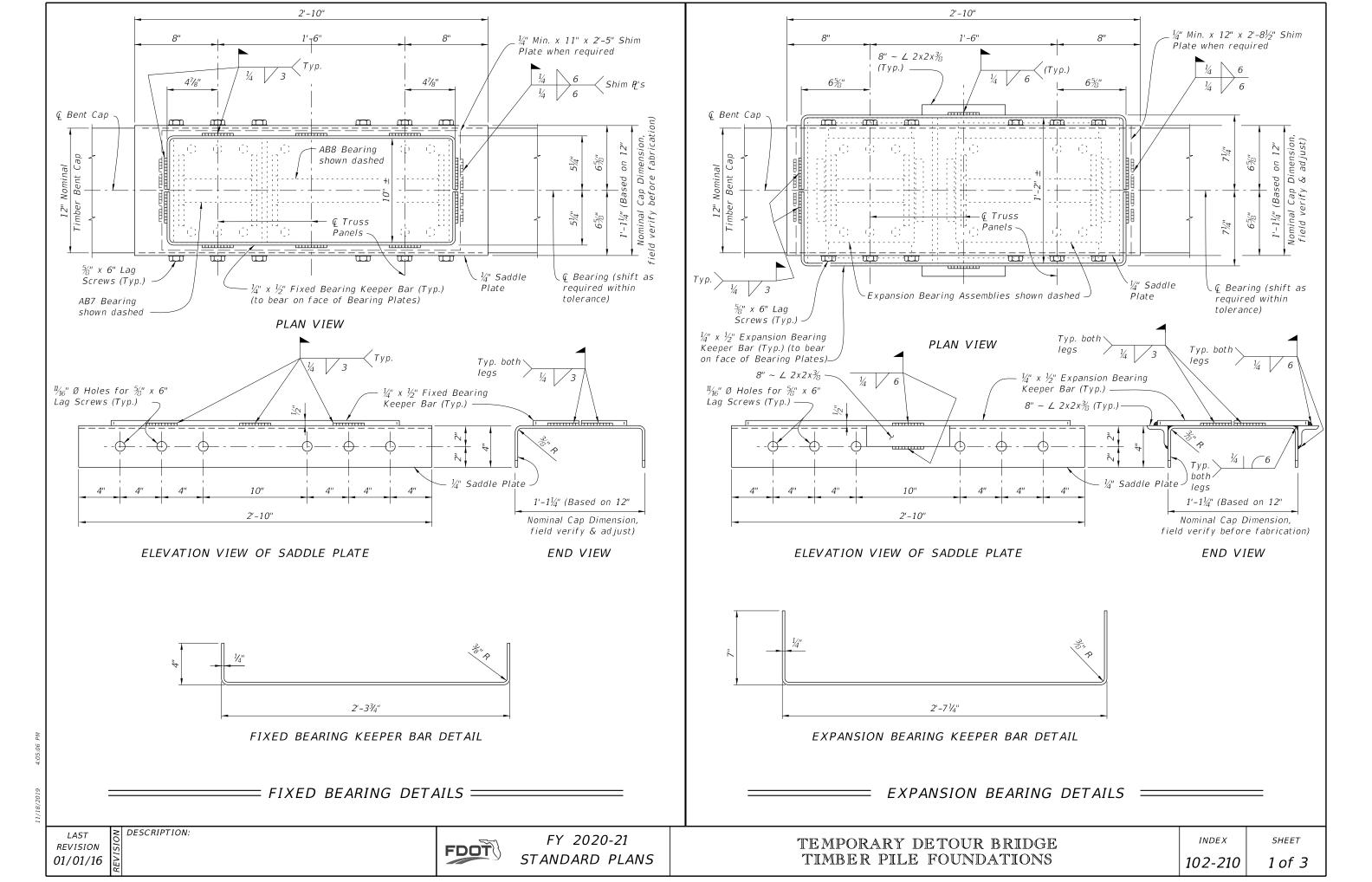
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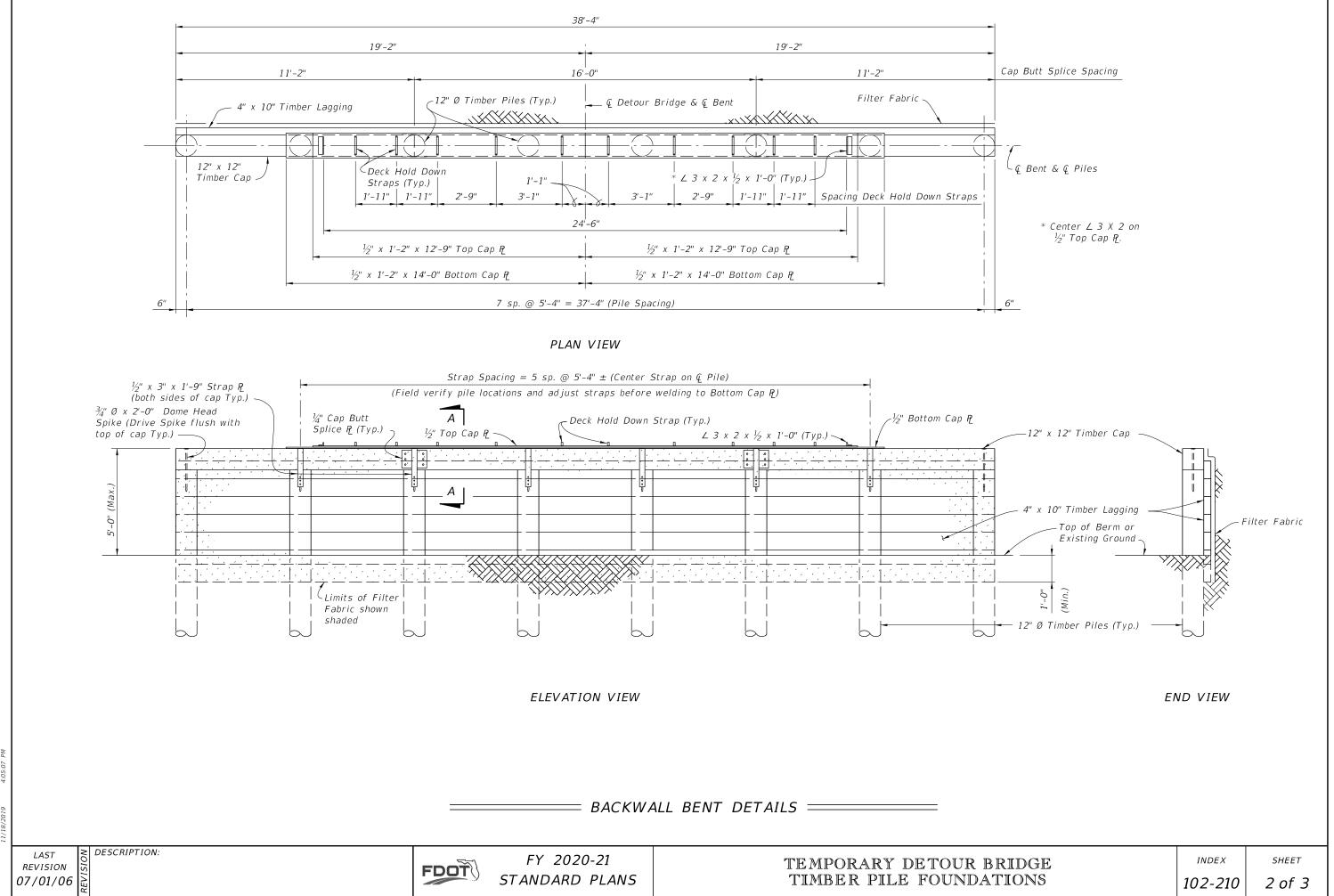


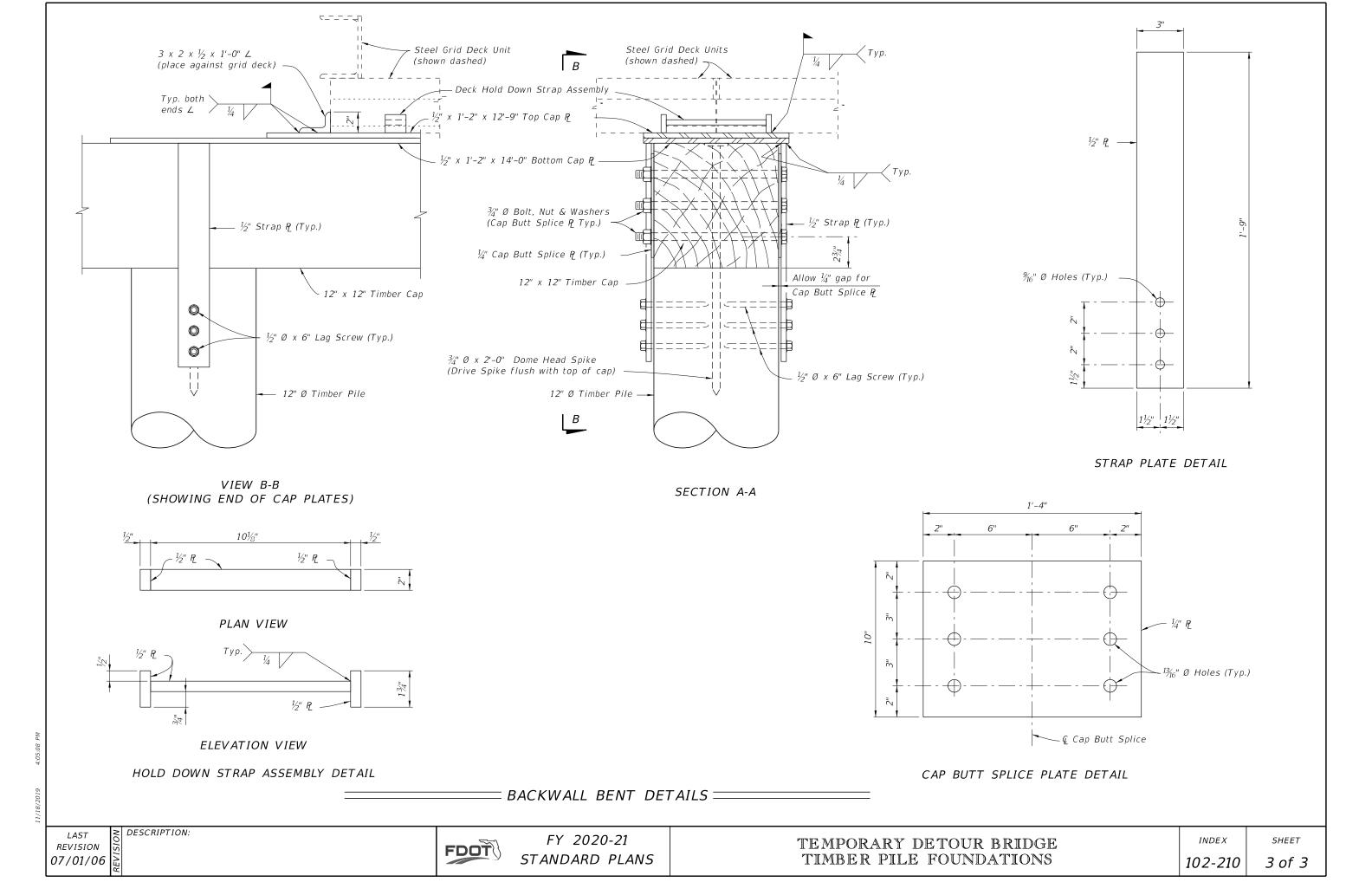


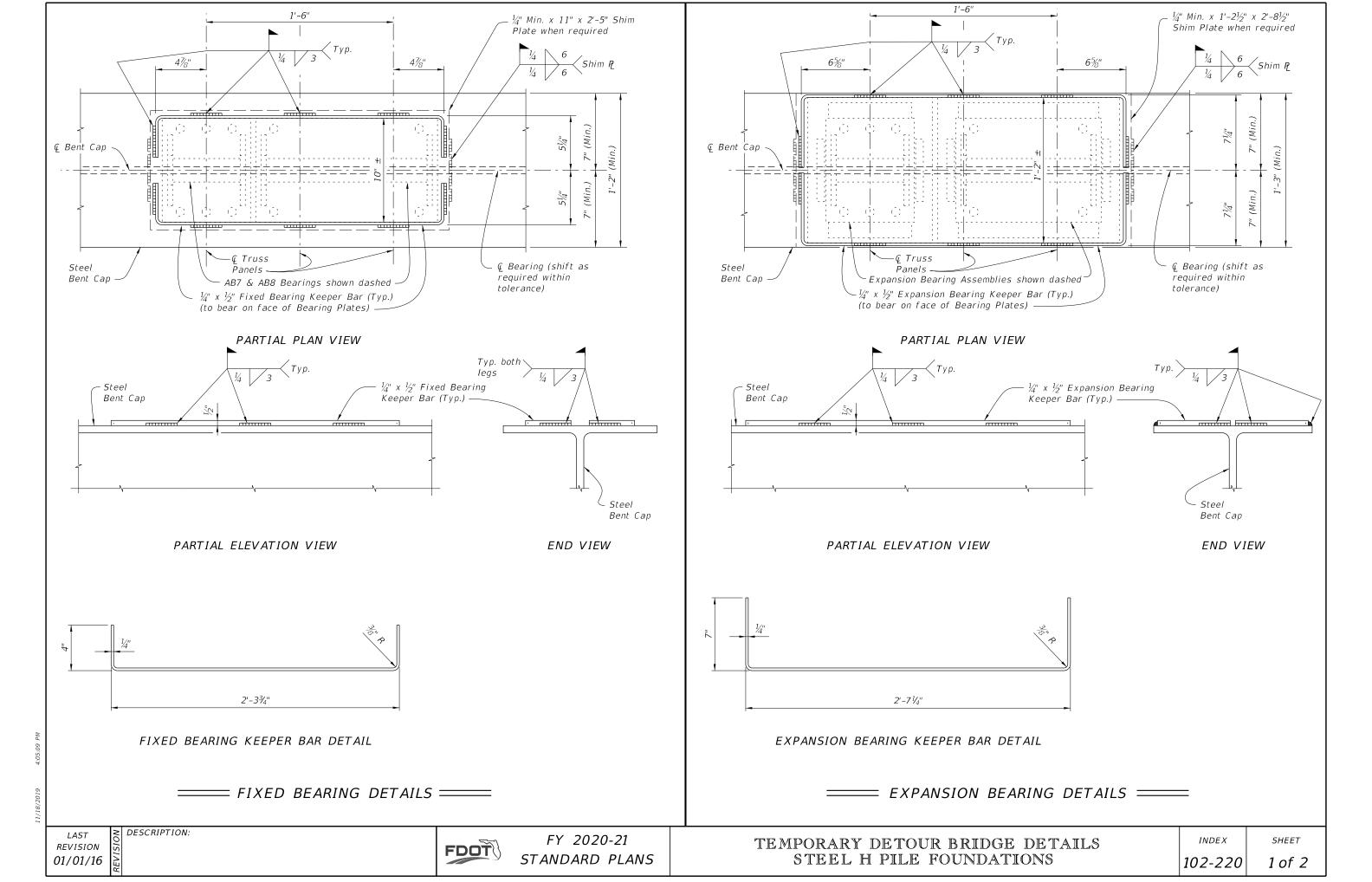


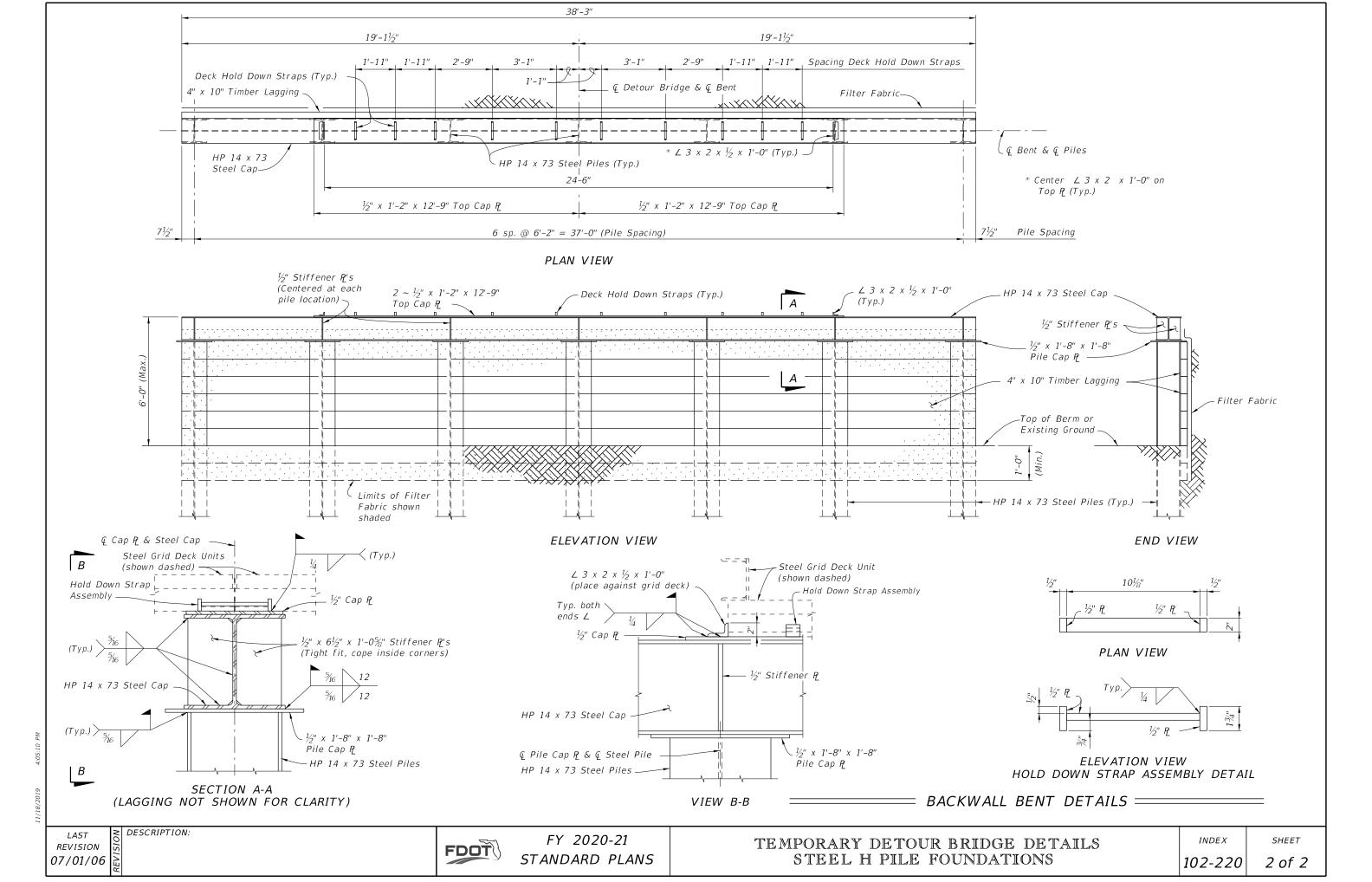


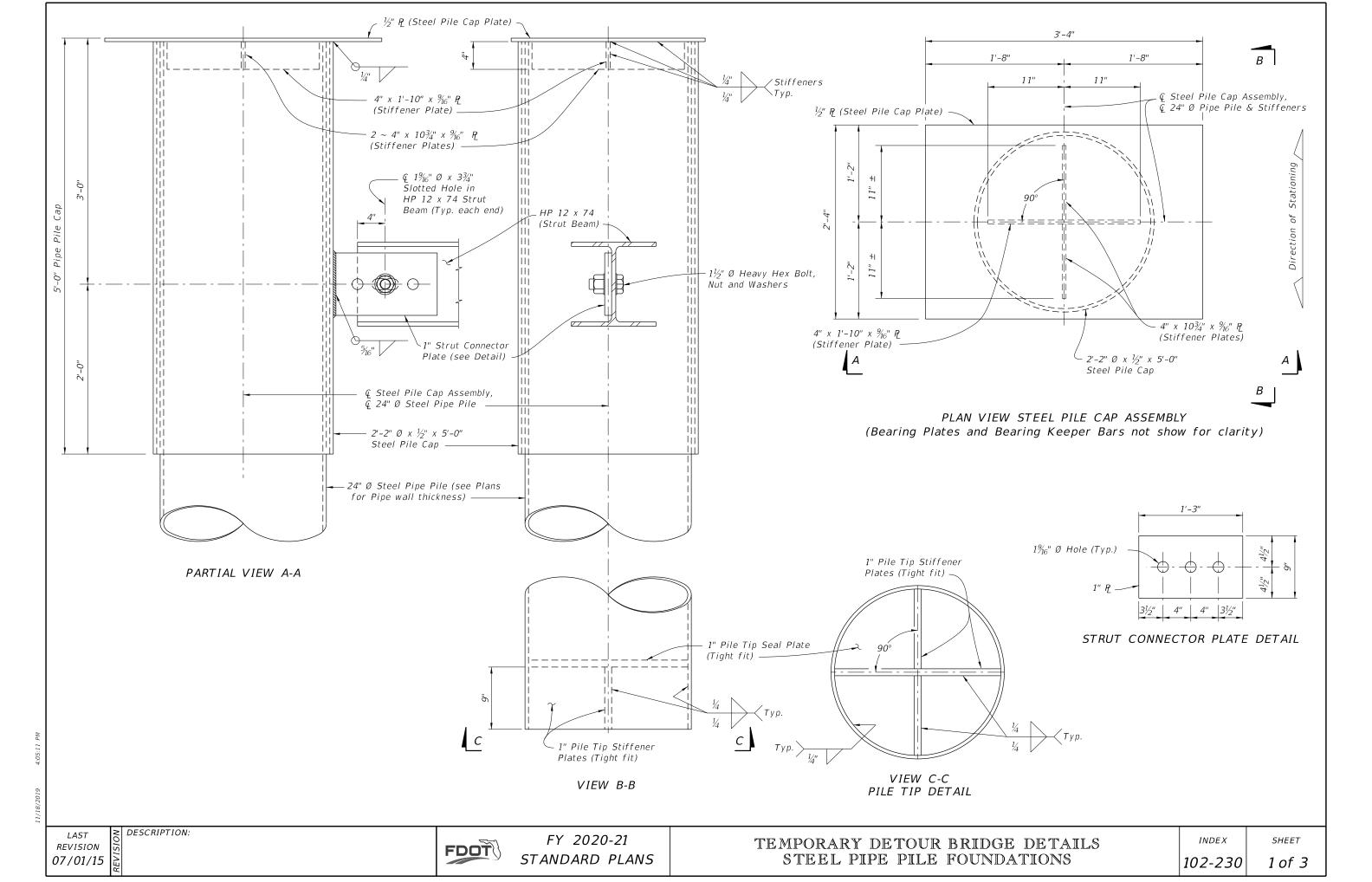


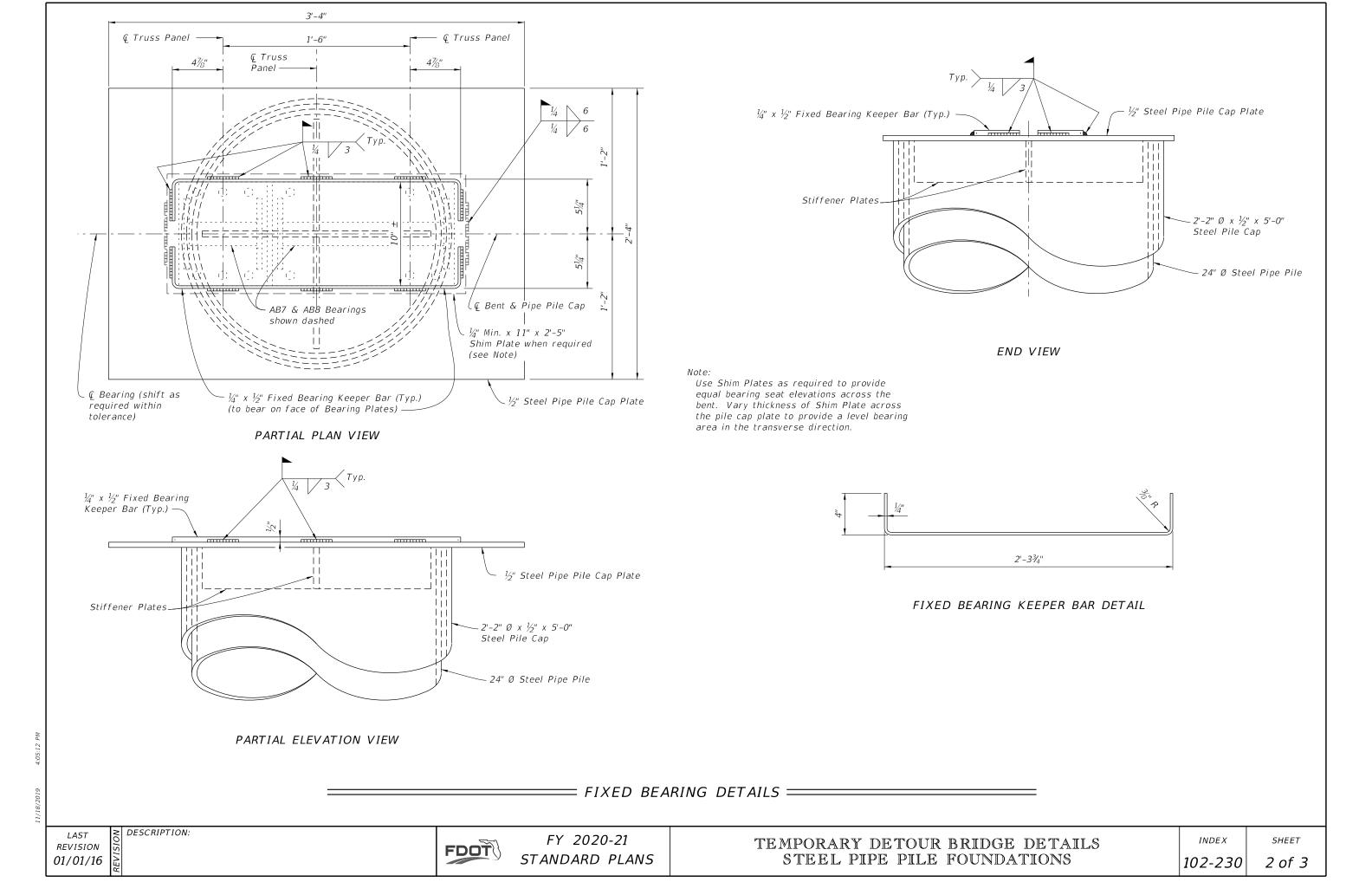


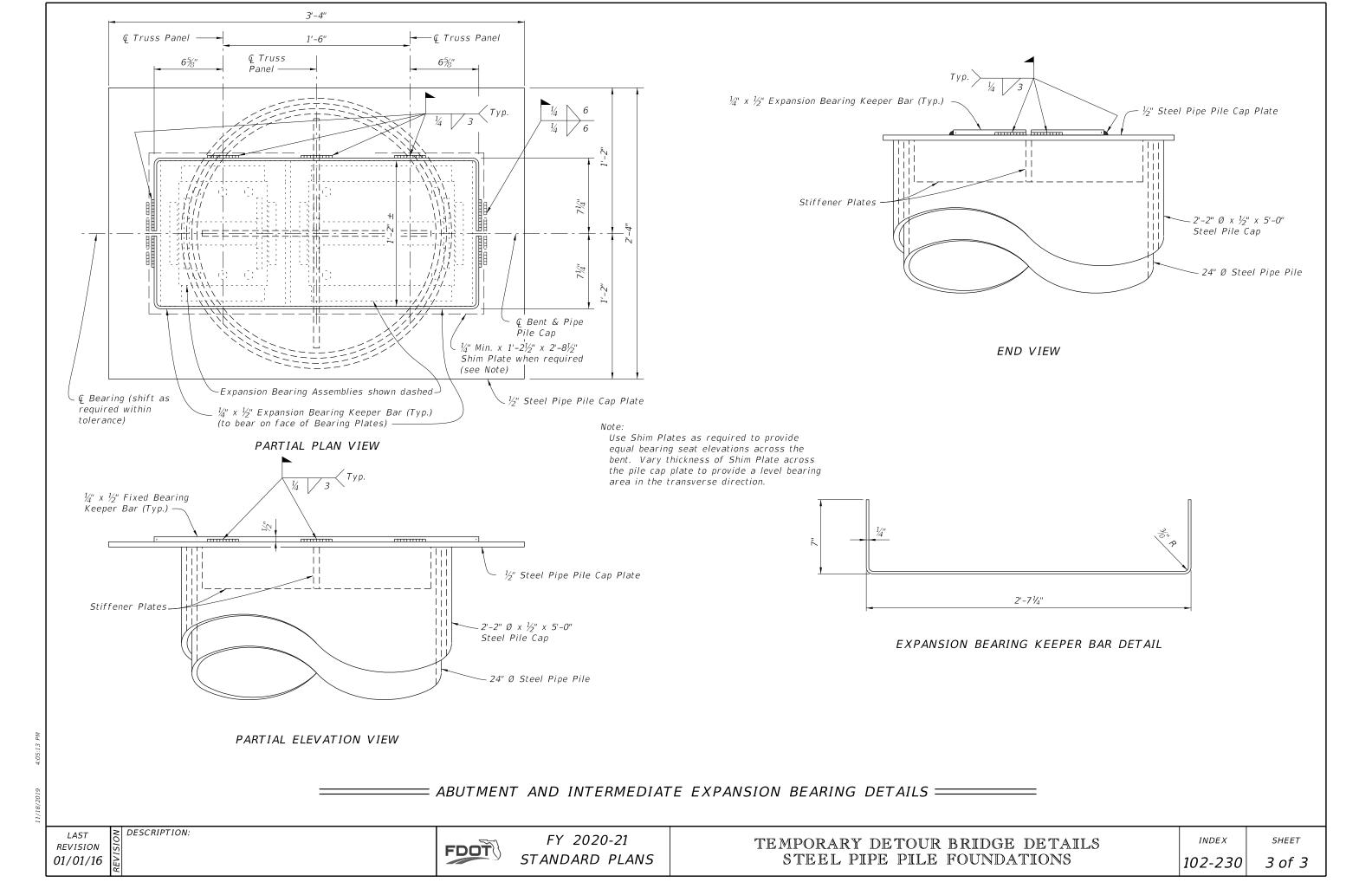


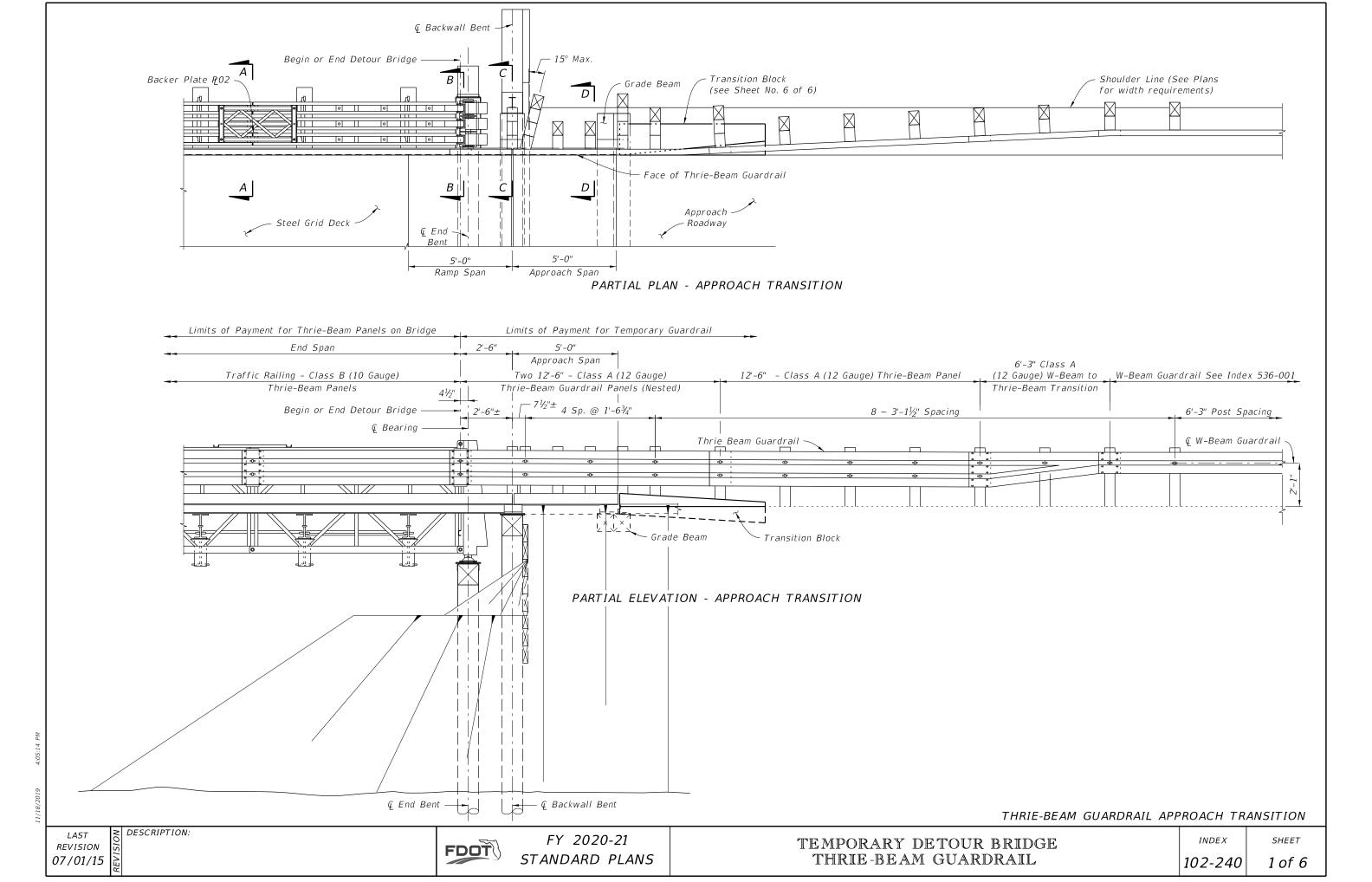


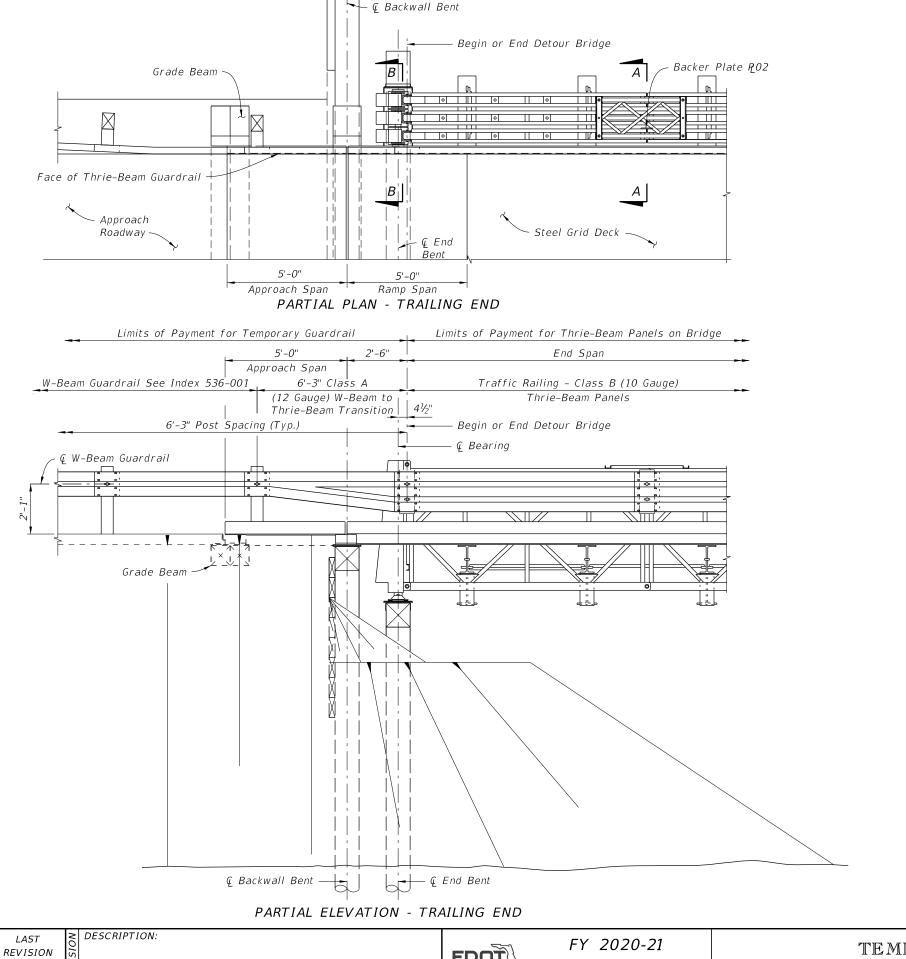


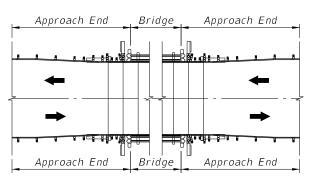




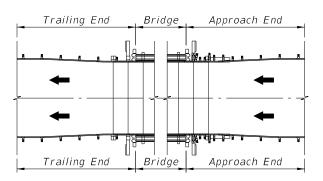








TWO-WAY TRAFFIC



ONE-WAY TRAFFIC END TRANSITION APPLICATION DETAILS

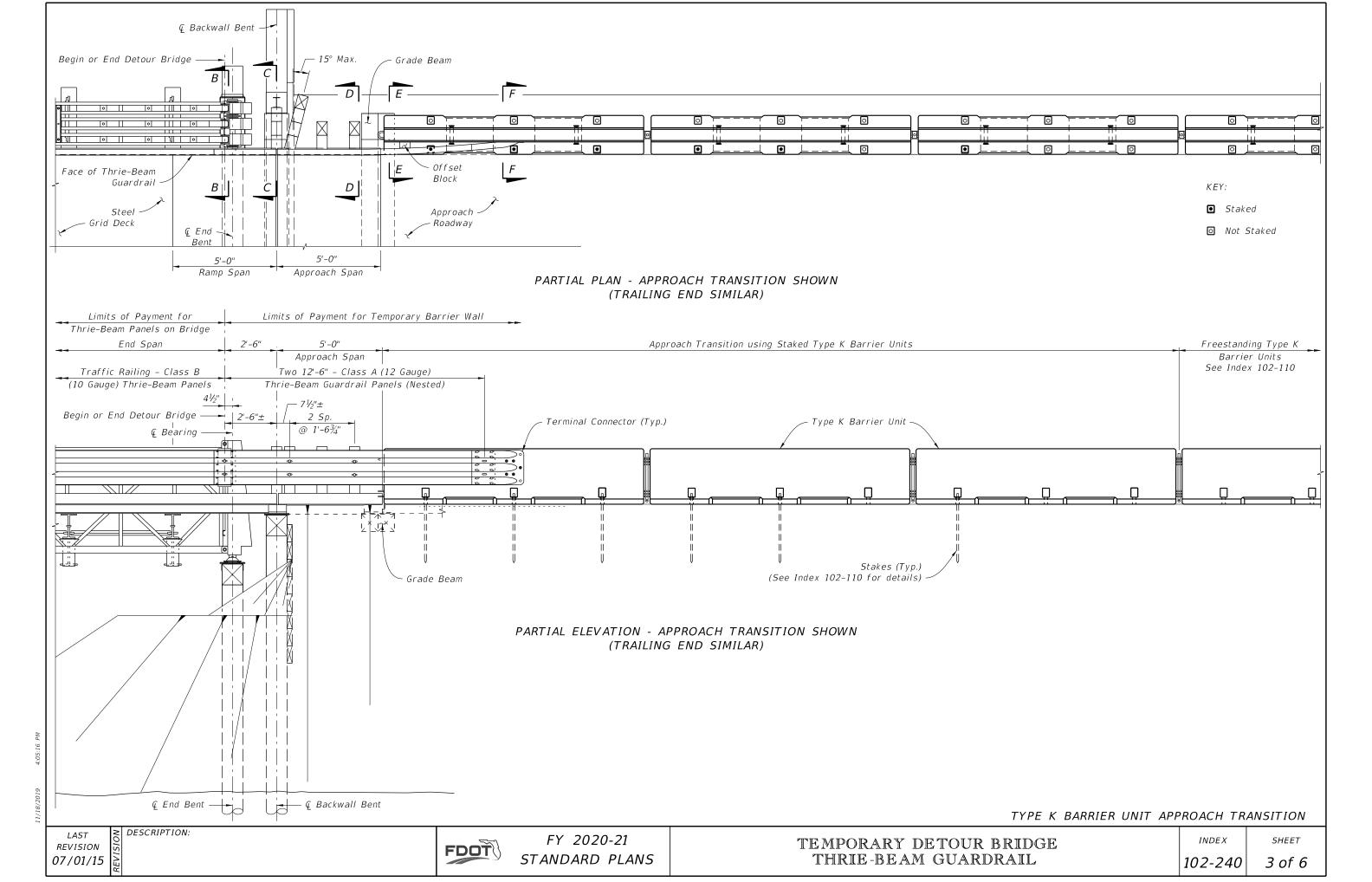
THRIE-BEAM GUARDRAIL TRAILING END TRANSITION

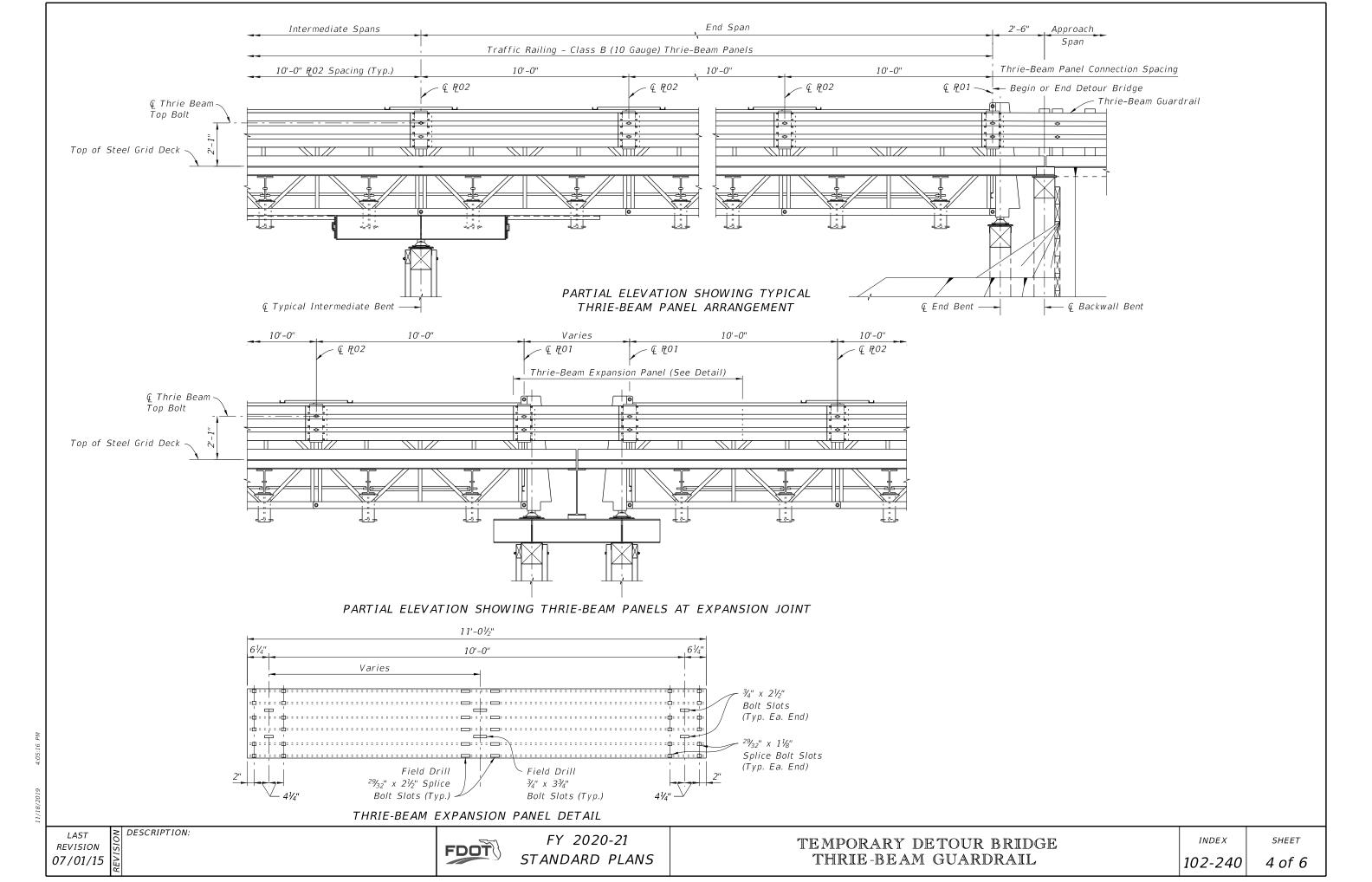
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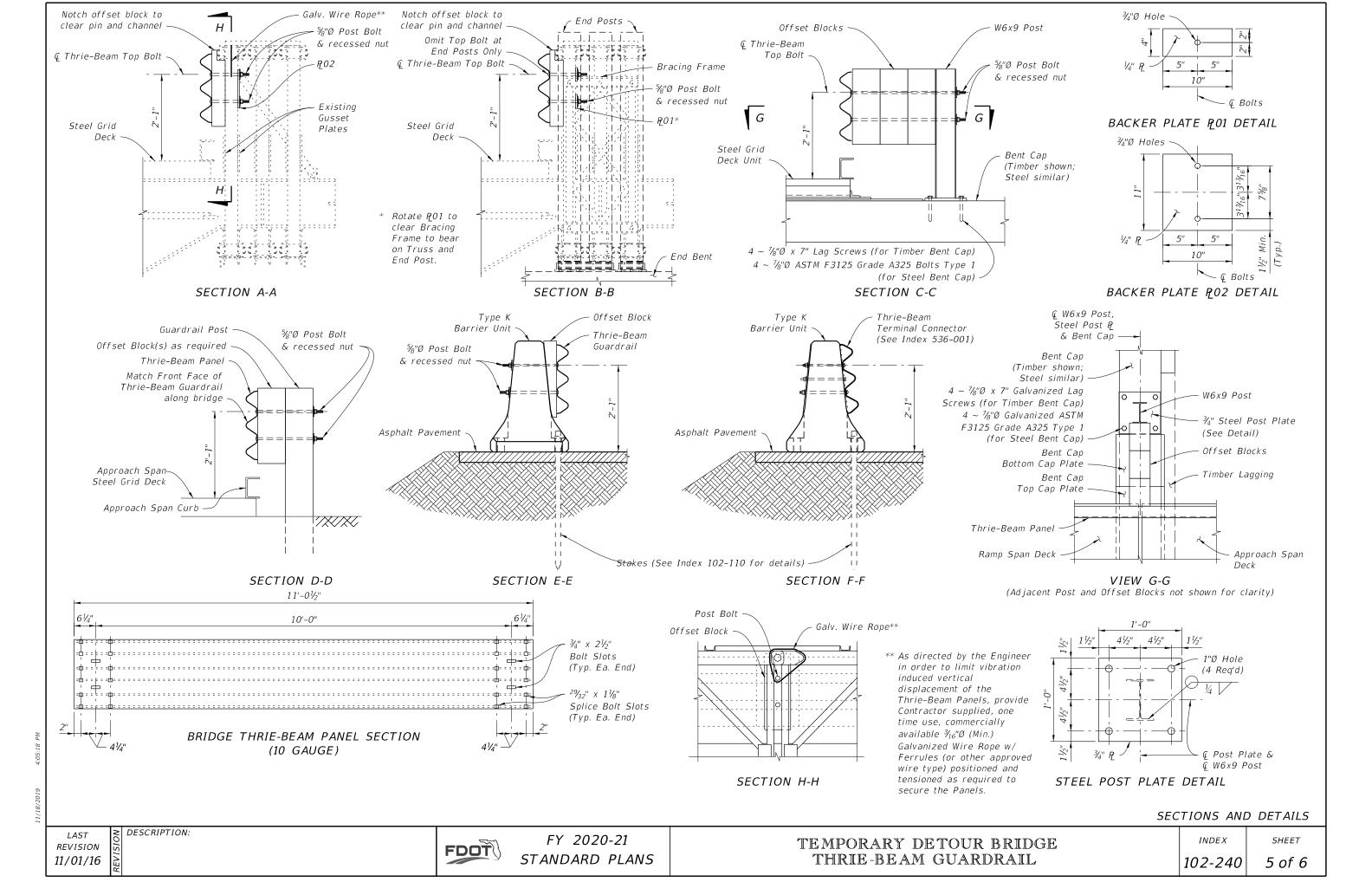
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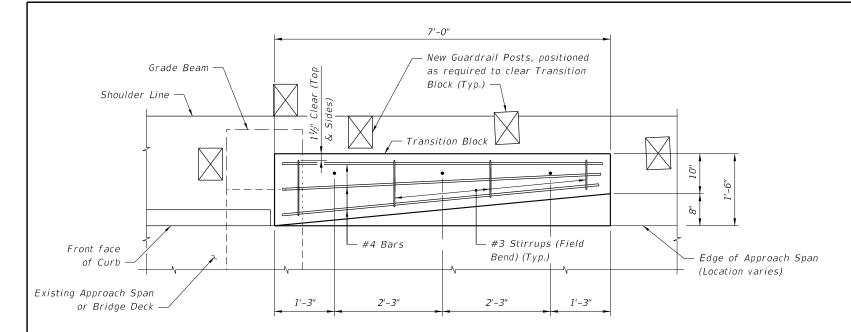
STANDARD PLANS

INDEX 102-240 SHEET

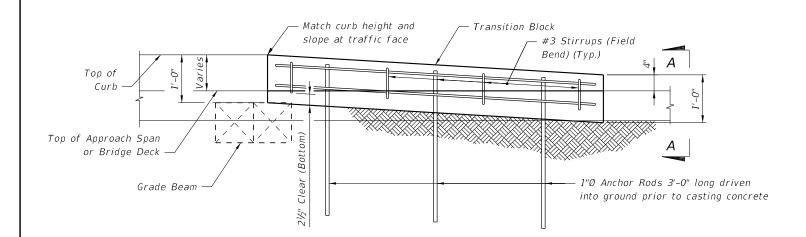






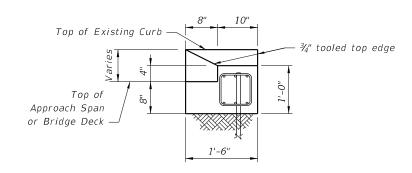


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

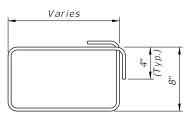


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUA	ANTITIE	S
ITEM	UNIT	QUANTITY
Concrete Class NS	CY	0.4
Reinforcing Steel	LB	61
Guardrail (Reset)	LF	12.5



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

LAST REVISION 07/01/13

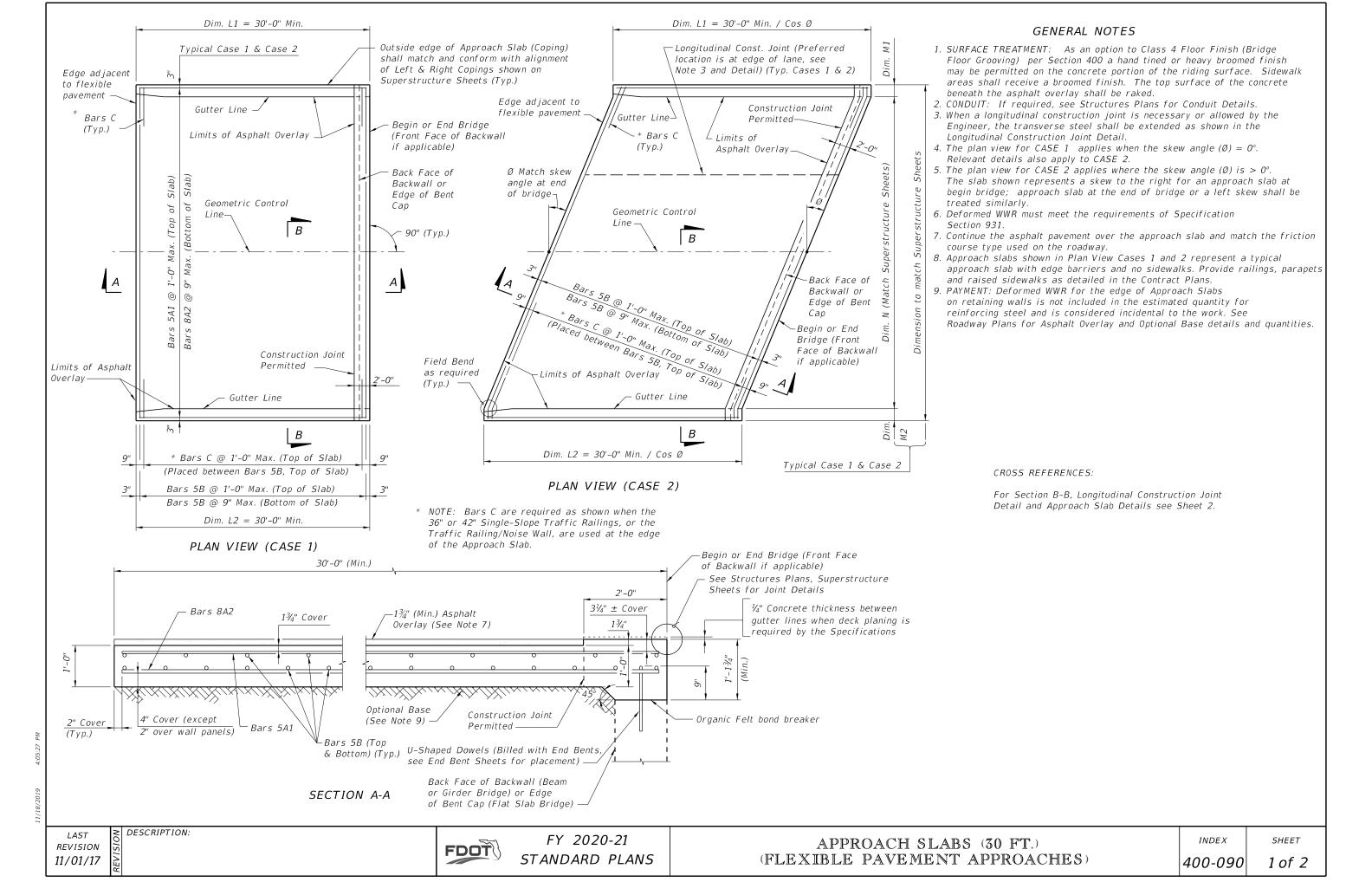
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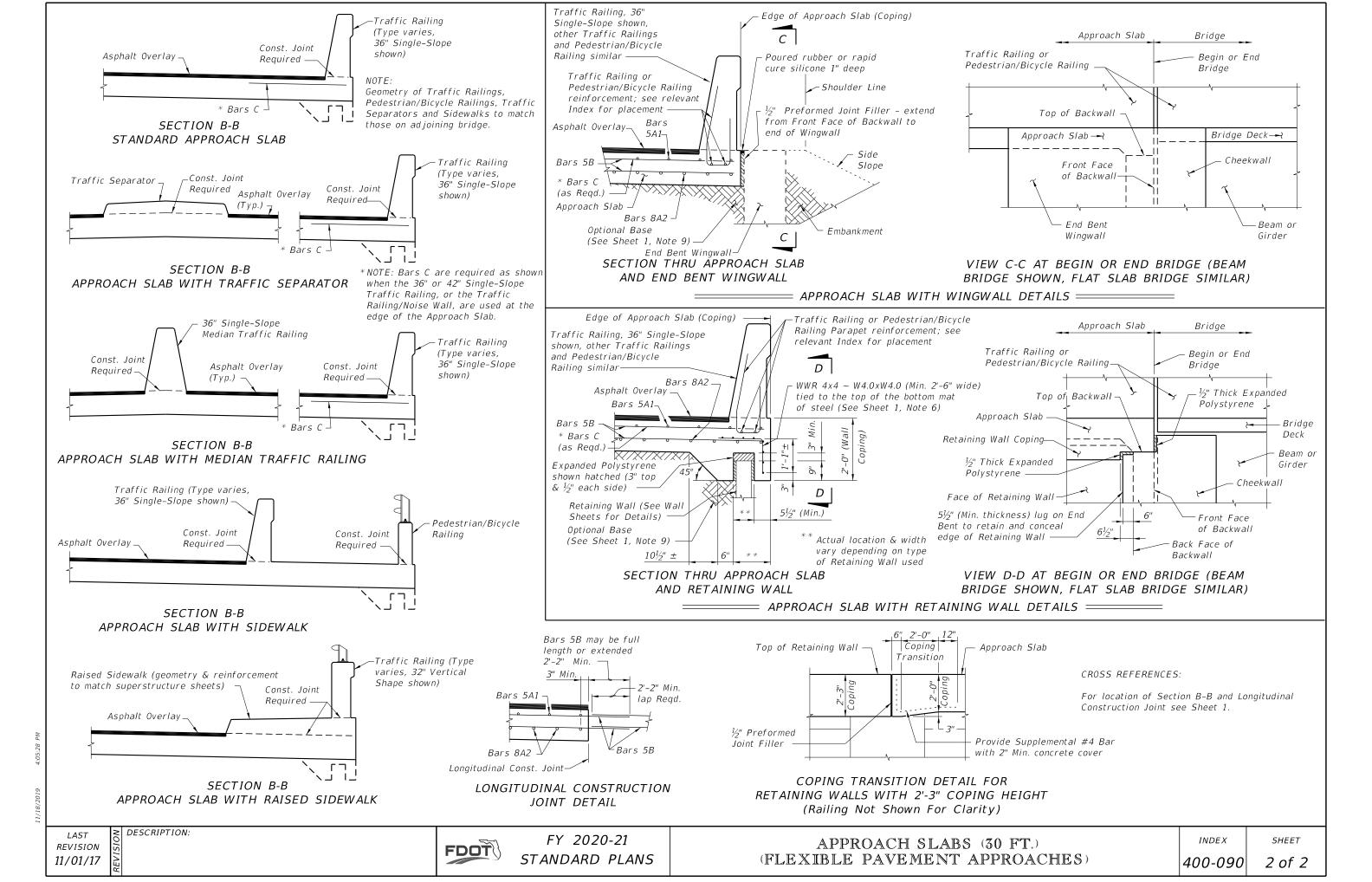
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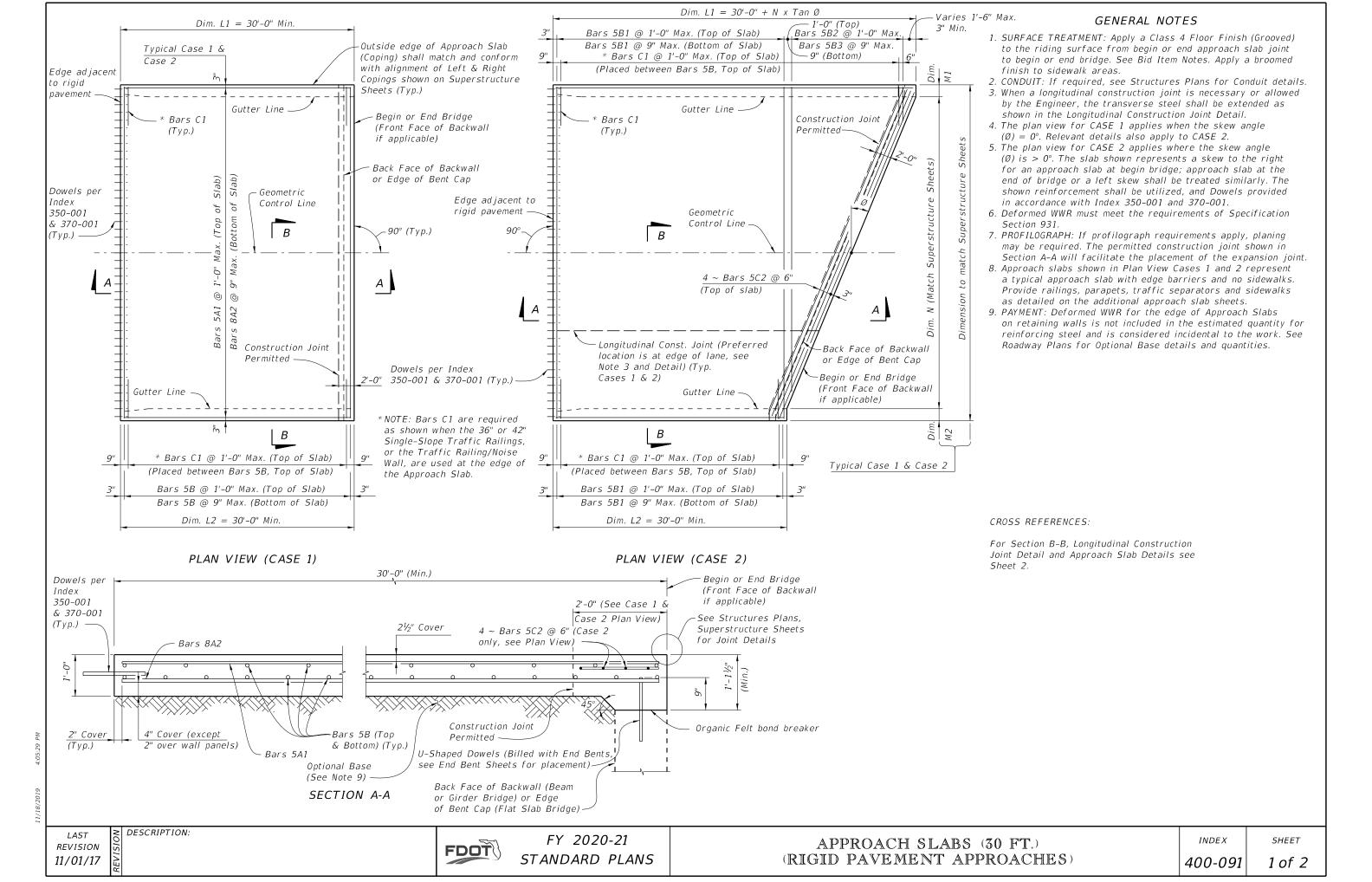
FY 2020-21 STANDARD PLANS INDEX 102-240

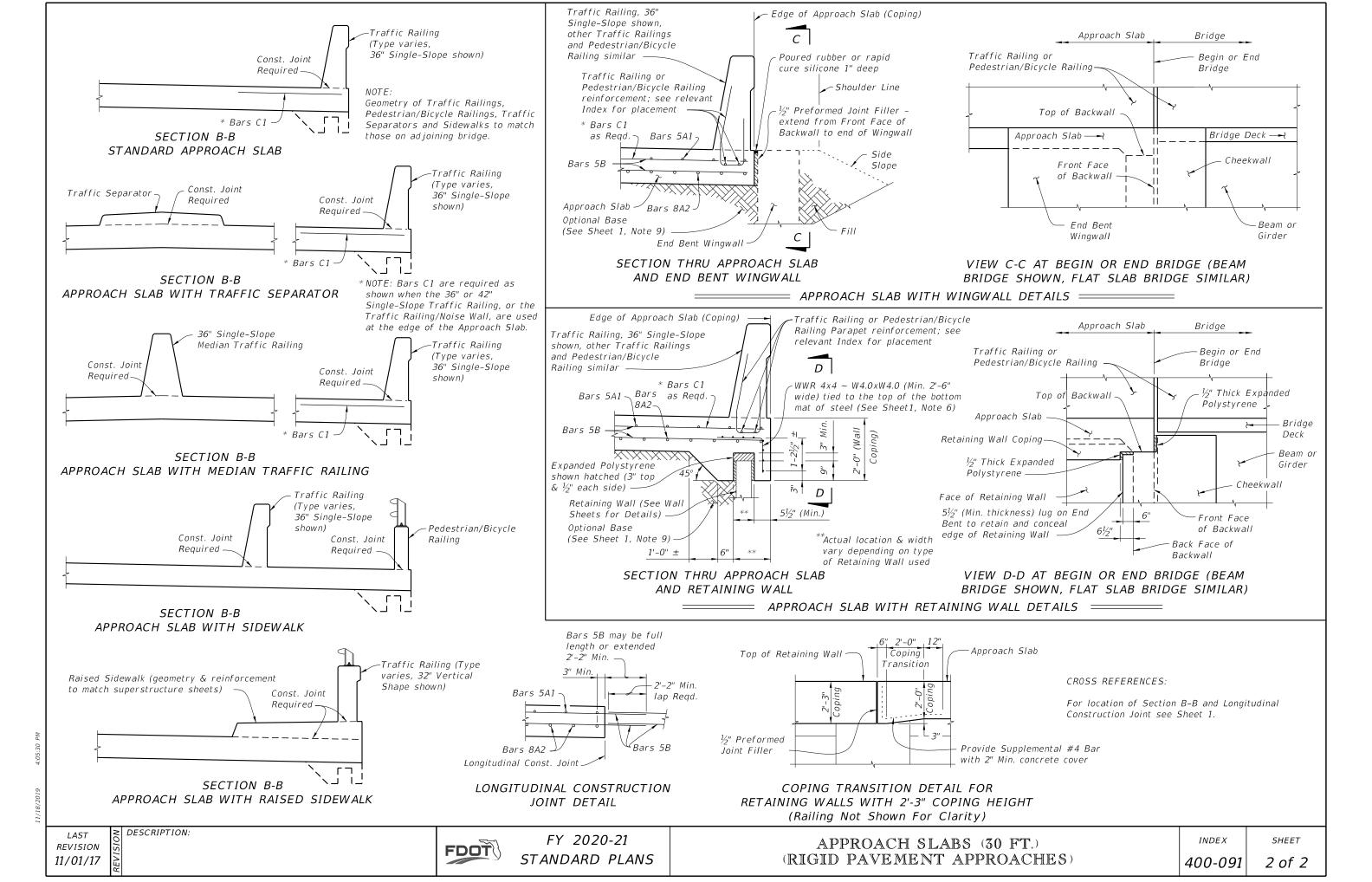
SHEET

11/18/2019

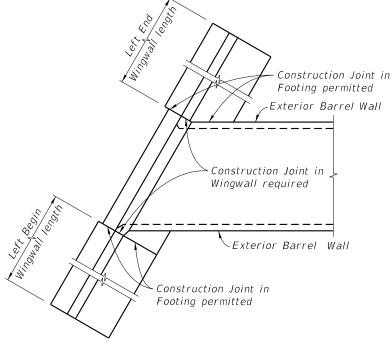








a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic "B".



PART PLAN SHOWING PARALLEL WINGWALLS AND LOCATION OF CONSTRUCTION JOINTS

NOTE:

DESCRIPTION:

Construction Joints in wingwalls and footings are located as follows: For non-skewed wingwalls they are located adjacent to the exterior face of the exterior barrel wall; when the Ç of wingwall and Ç of exterior barrel wall results in an acute angle see Left End Wingwall above, and when the angle is obtuse see Left Begin Wingwall above and Detail C (Sheet 5).

GENERAL NOTES:

LIVE LOAD: HL-93.

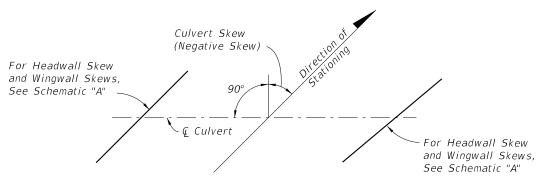
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 8.

REINFORCING STEEL: See the "Box Culvert Data Tables" in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.

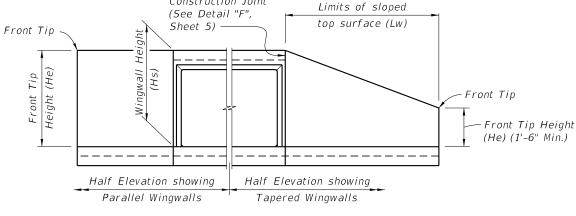


SCHEMATIC "B" - PLAN VIEW CULVERT ALIGNMENT

NOTE: For Culvert Skew see Contract Plans.

TAB	LE 1 - MI	NIMUM E	BAR SP	LICE LEN	GTHS
	FOR LON	IGITUDIN.	AL REI	NFORCIN	G
BAR	SPLICE (CLASS B)	BAR	SPLICE (CLASS B)
SIZE	CLASS II	CLASS IV	SIZE	CLASS II	CLASS IV
	(3400 psi)	(5500 psi)		(3400 psi)	(5500 psi)
#3	1'-4"	1'-0"	#8	3'-5"	2'-8"
#4	1'-9"	1'-4"	#9	4'-3"	3'-4"
#5	2'-2"	1'-8"			
#6	2'-7"	2'-0"			
#7	3'-0"	2'-4"			

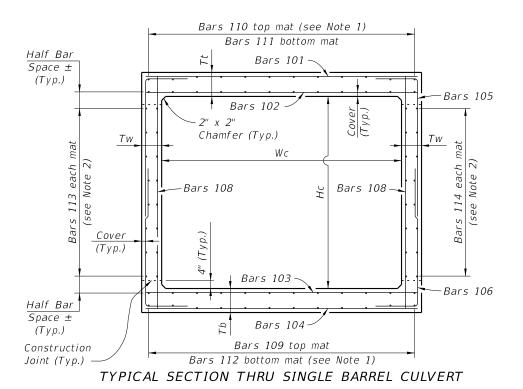
TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.



END ELEVATION OF CULVERT

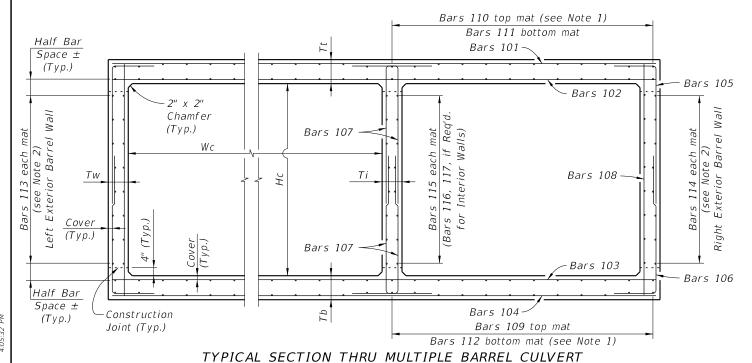
Construction Joint

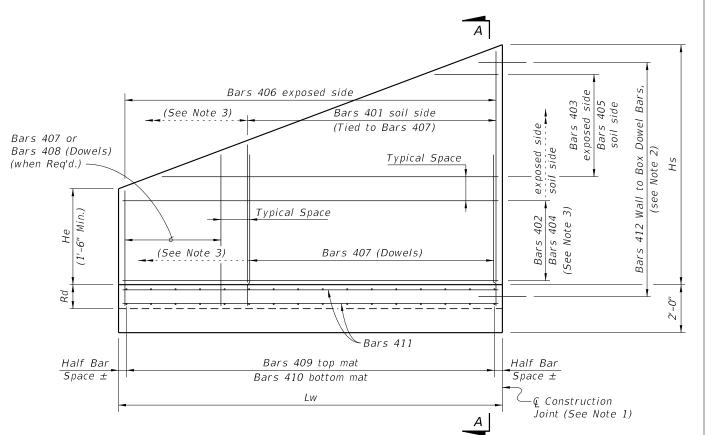
REVISION 11/01/16



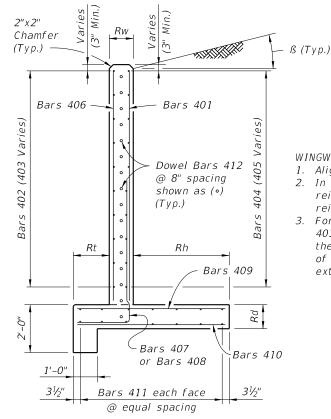
CULVERT BARREL NOTES:

- 1. Space Bars 110 and 112 with a bar in each corner, and at the $\c Q$ of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4" clear.





WINGWALL ELEVATION - Variable Height (Left End shown - other corners similar)



WINGWALL NOTES:

- 1. Align construction joint perpendicular to wingwall.
- 2. In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover.
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

WINGWALL SECTION A-A

DESCRIPTION: REVISION 07/01/13

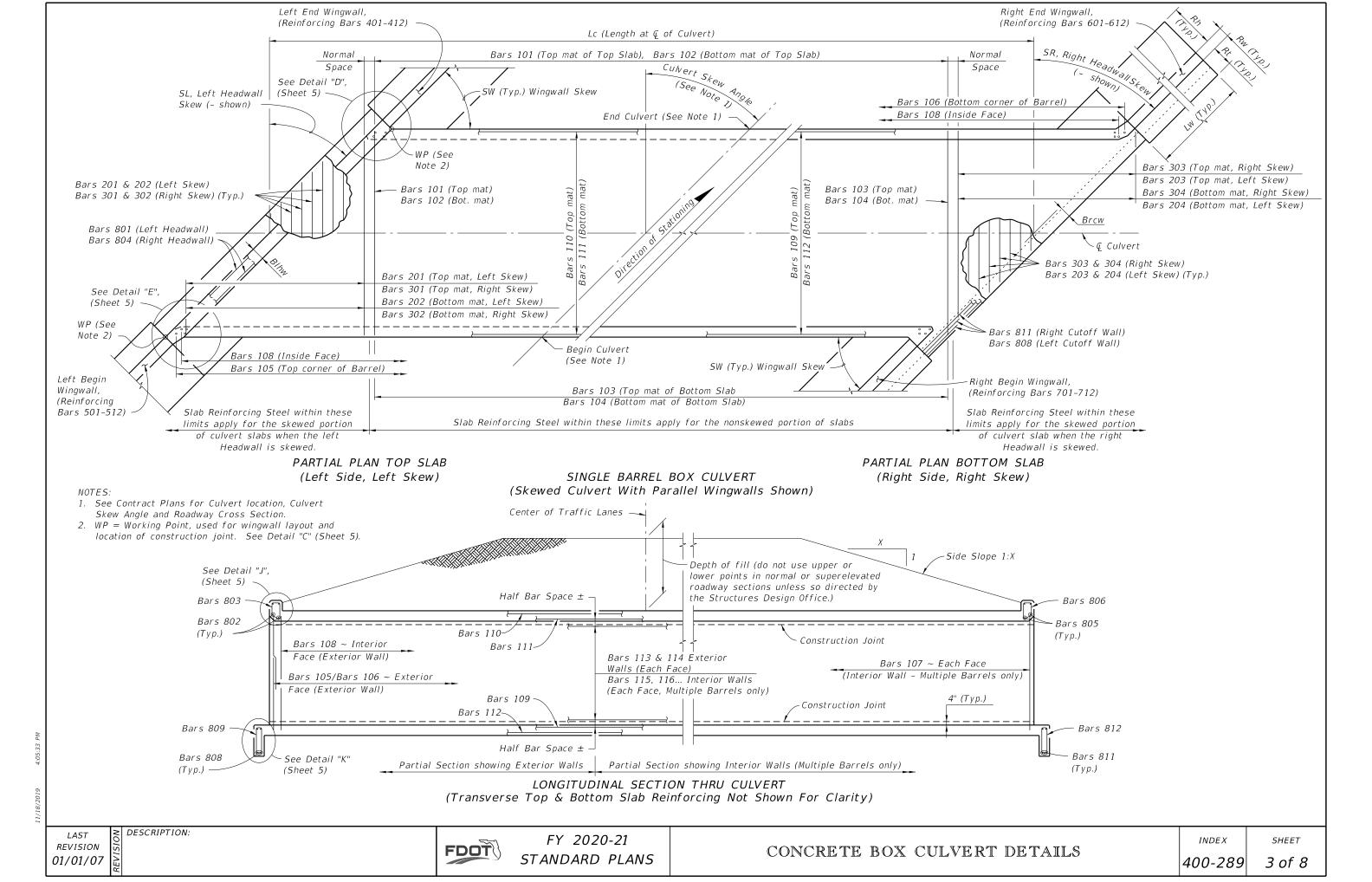
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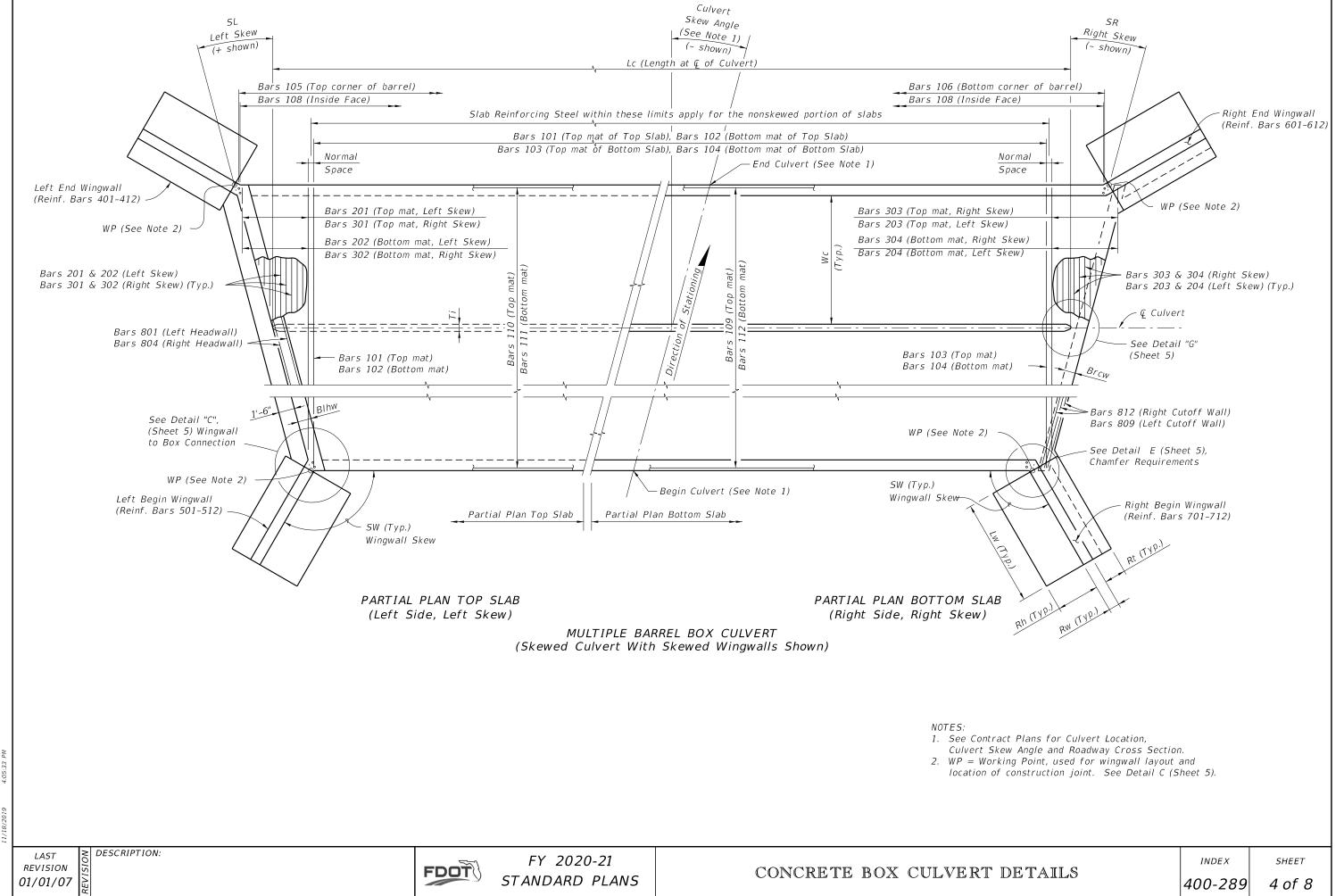
FY 2020-21 STANDARD PLANS

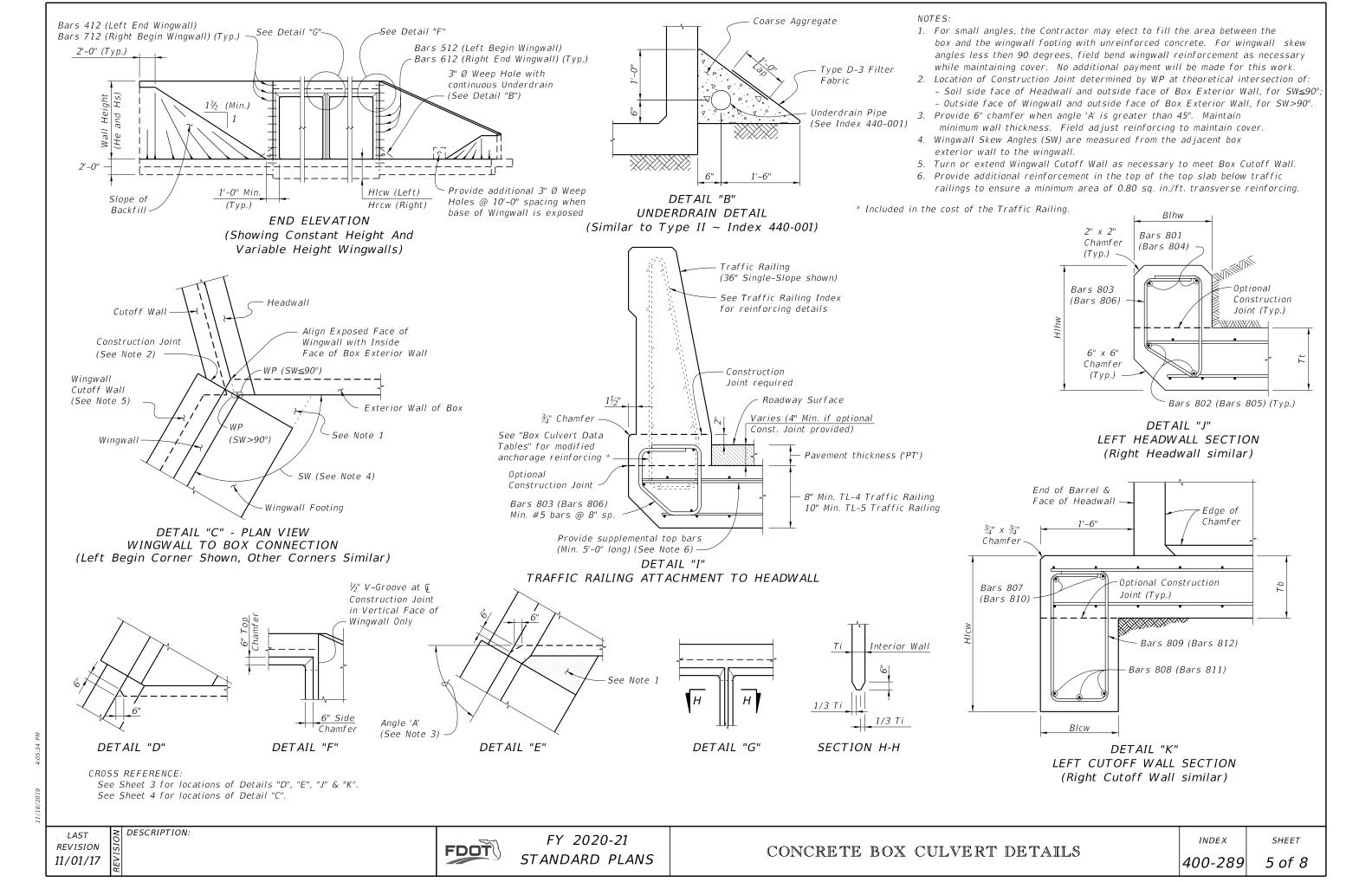
CONCRETE BOX CULVERT DETAILS

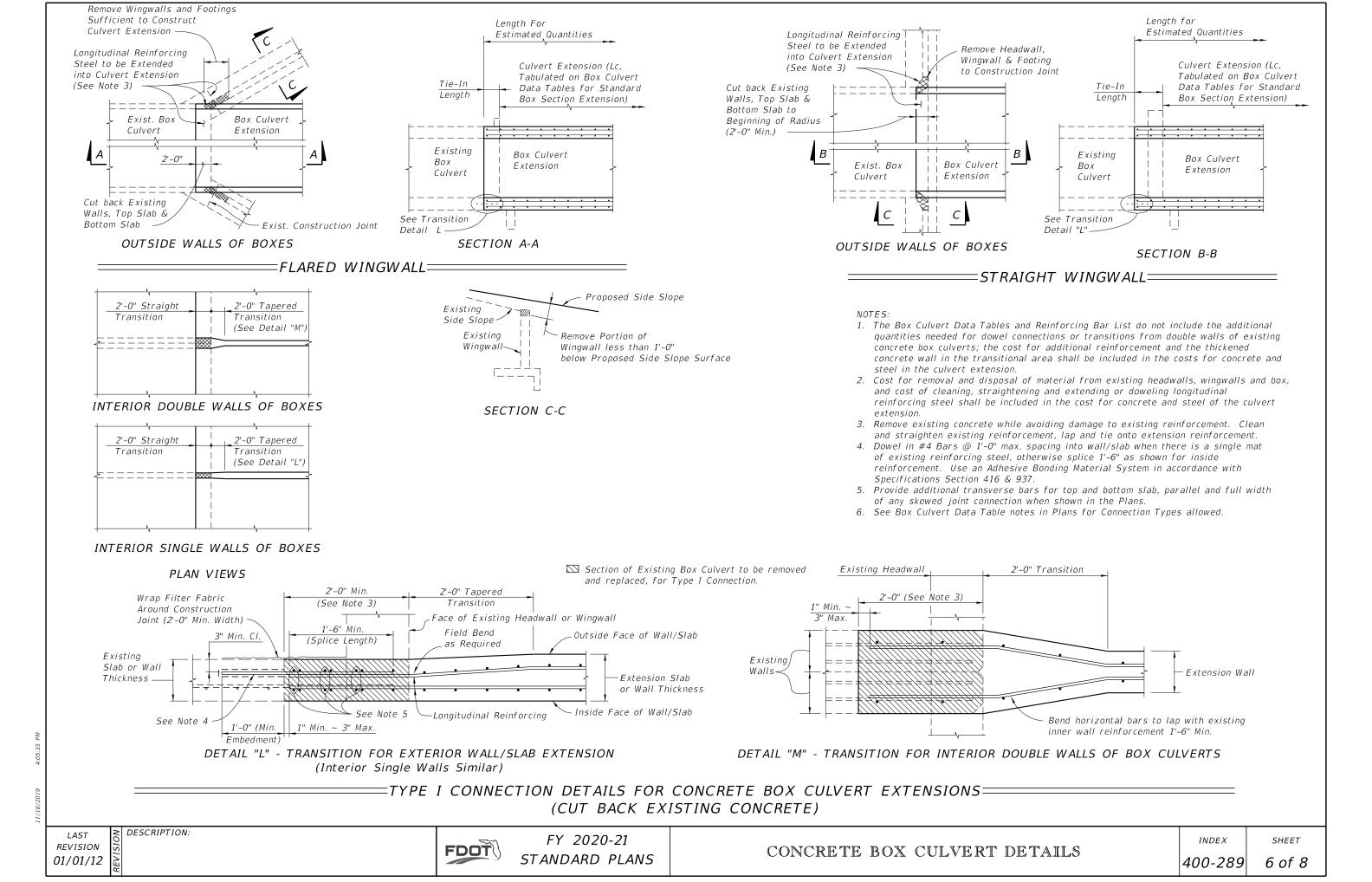
SHEET INDEX 400-289

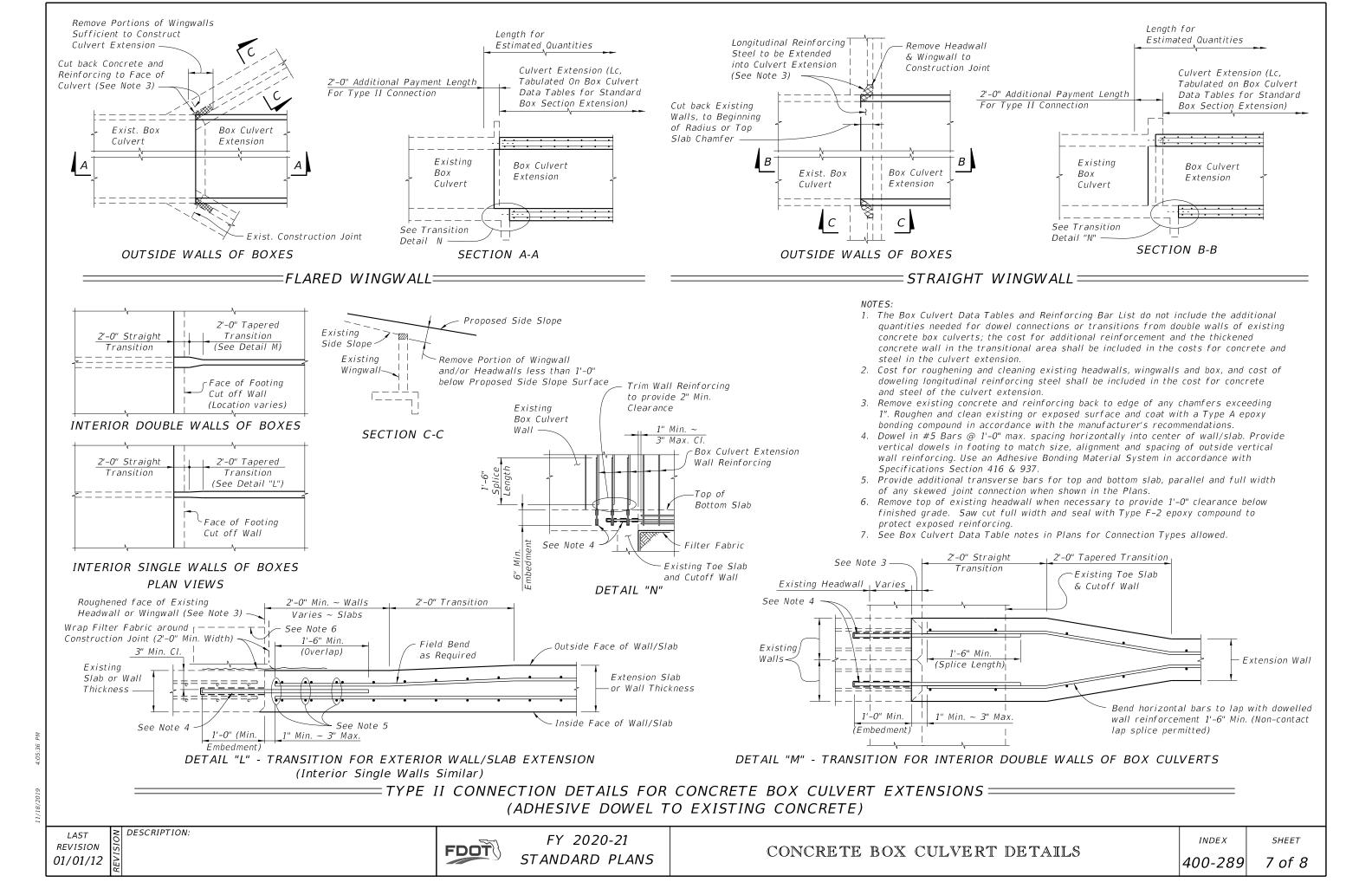
2 of 8

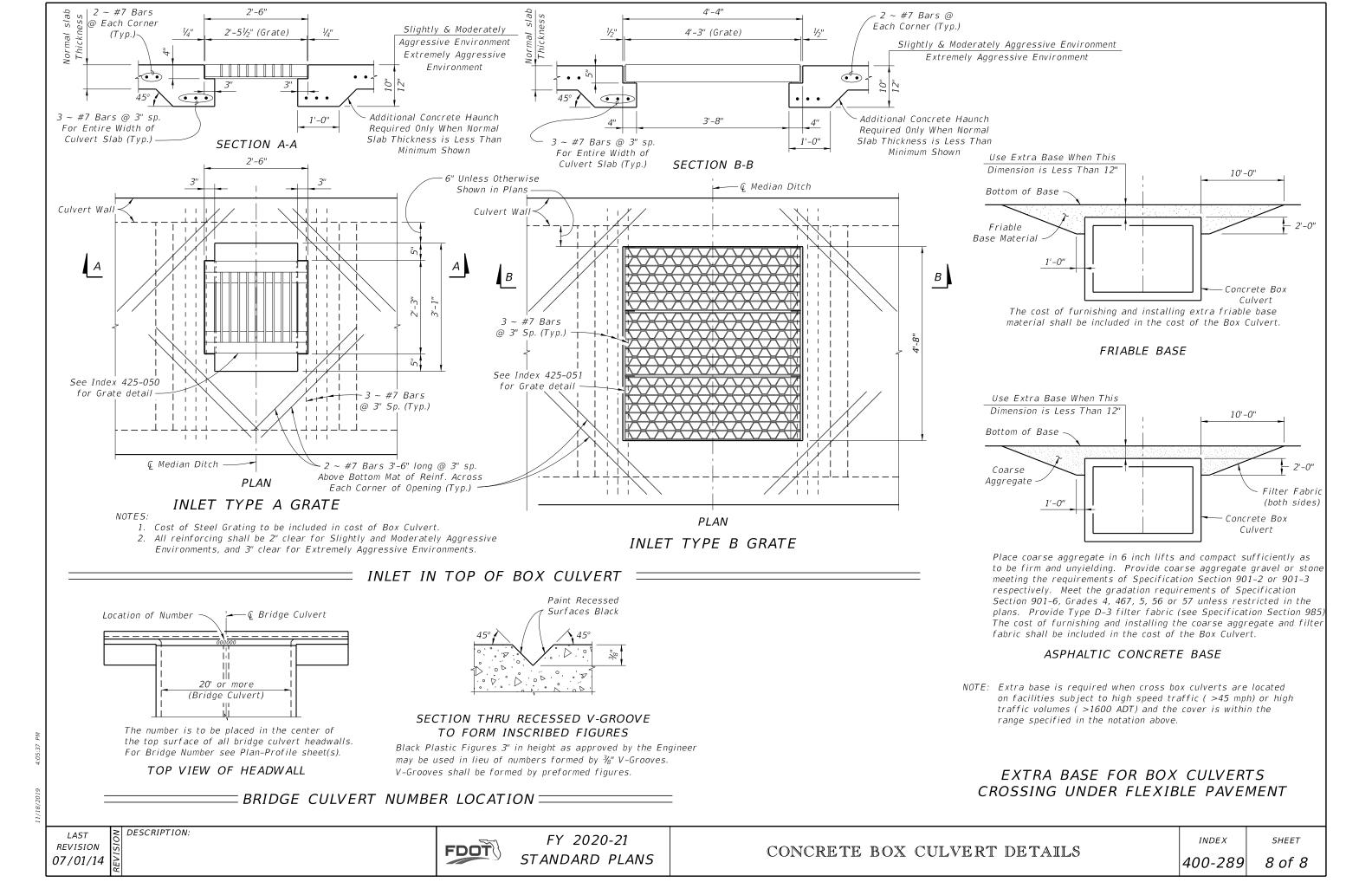


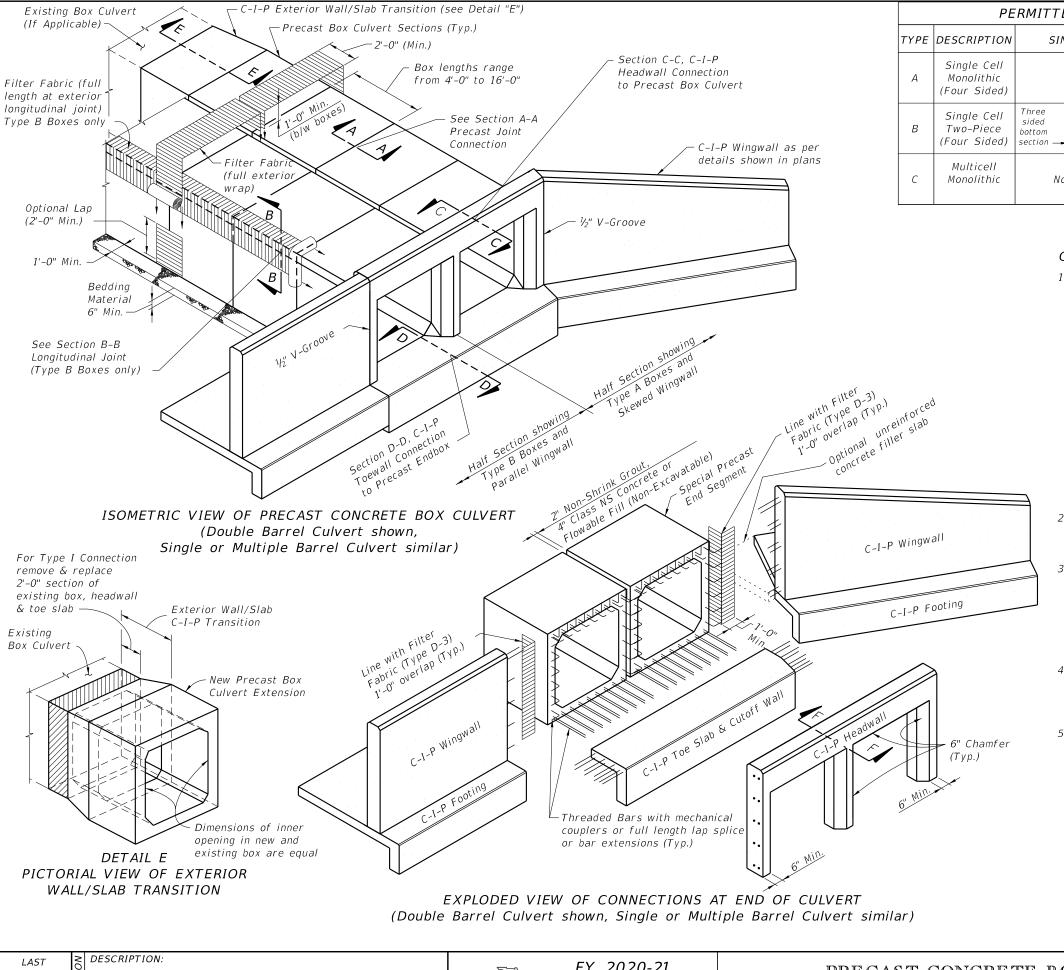












PERMITTED PRECAST ALTERNATE BOX SECTIONS SINGLE BARREL MULTIPLE BARRELS DESIGN NOTES Index 400-292 Contractor Design section Contractor Design Contractor Design Not Applicable

GENERAL NOTES:

1. Specifications:

FDOT Standard Specifications for Road and Bridge Construction, Section 410 (current edition, and supplements thereto).

Concrete (Precast):

Class III or Class II Modified (5,000 psi) for slightly aggressive environments.

Class IV (5,500 psi) for moderately to extremely aggressive environments.

Concrete (Cast-In-Place):

Class II (3,400 psi) for slightly aggressive environments. Class IV (5,500 psi) for moderately to extremely aggressive environments.

Reinforcing Steel:

Maintain minimum clearance of 2" for slightly and moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. Equal area substitution of welded wire (WWR) reinforcement is permitted.

- 2. Work this Index with the Cast-In-Place Concrete Box Culvert Details and Data Tables shown in the plans, Index 400-289 and the Precast Concrete Box Culverts shown in the shop drawings.
- 3. All joints between precast sections must be tongue & groove with joint sealant. Joints between cast-in-place & precast sections shall have longitudinal reinforcing extending from top, bottom & both side slabs of the precast box tied to the cast-in-place reinforcement. Single barrel culverts may have precast headwalls cast integrally with the end segment when approved by the Engineer.
- 4. Extension of existing multiple barrel box culverts with multiple single cell precast box culverts is not permitted unless approved by the District Structures Engineer. Full transition details must be shown in the shop drawings when approved.
- 5. Culverts larger than the specified size may be substituted with no additional payment to the Contractor. Substitution must be approved by the Engineer, minimum earth cover and invert elevations shown in the Contract Documents must be maintained.

REVISION 01/01/11

FDOT

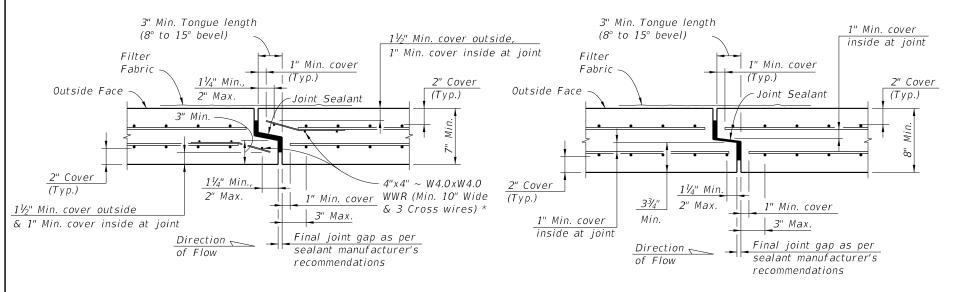
FY 2020-21 STANDARD PLANS

PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

INDEX

SHEET

400-291



SECTION A-A (2" Cover - Thin Wall Detail)

* At the Contractor's option when the box culvert reinforcing utilizes WWR, extend wall and slab

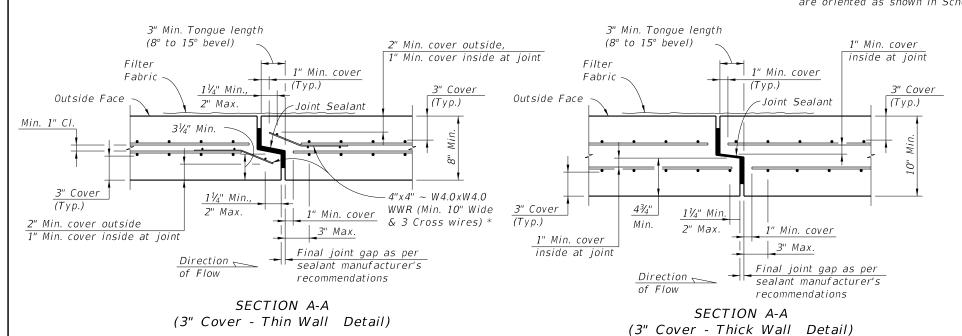
reinforcing into the joint and bend to maintain cover in lieu of 4"x4" ~ W4.0xW4.0 WWR at joint.

of box to allow bending of the WWR.

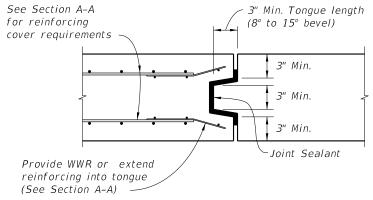
Transverse wire in tongue may be cut at corners

SECTION A-A (2" Cover - Thick Wall Detail)

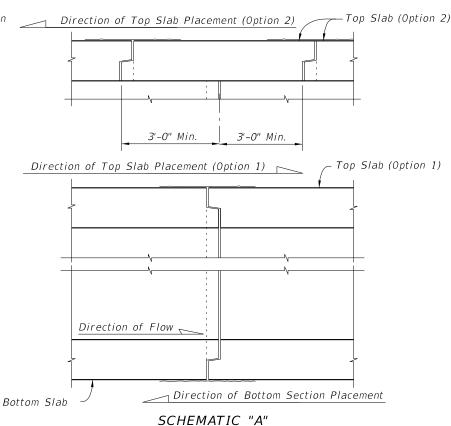
> NOTE: Bottom Slab Joints in Type B Boxes may be single tongue & groove joints as shown in Section A-A when the Top Slab Joints are oriented as shown in Schematic "A".



PRECAST SEGMENT TO SEGMENT TONGUE & GROOVE TRANSVERSE JOINTS ==



ALTERNATE BOTTOM SLAB TRANSVERSE JOINT TYPICAL SECTION (DOUBLE-SIDED TONGUE & GROOVE JOINT) (All reinforcing not shown for clarity)



TYPE B BOX SECTION PLACEMENT FOR SINGLE TONGUE & GROOVE JOINTS

= TWO-PIECE PRECAST SEGMENT ADDITIONAL JOINT DETAILS (TYPE B BOX)

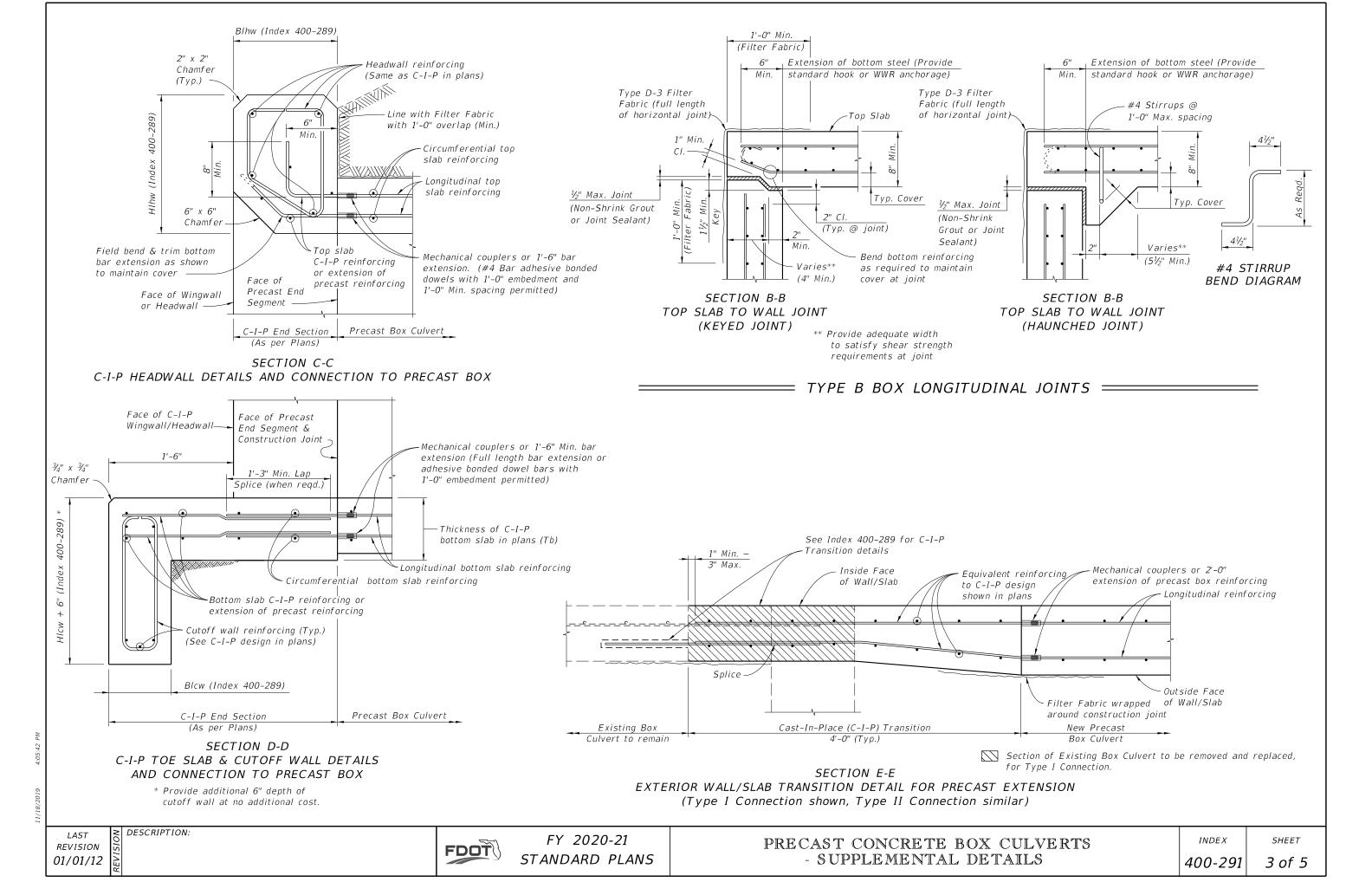
REVISION 07/01/15

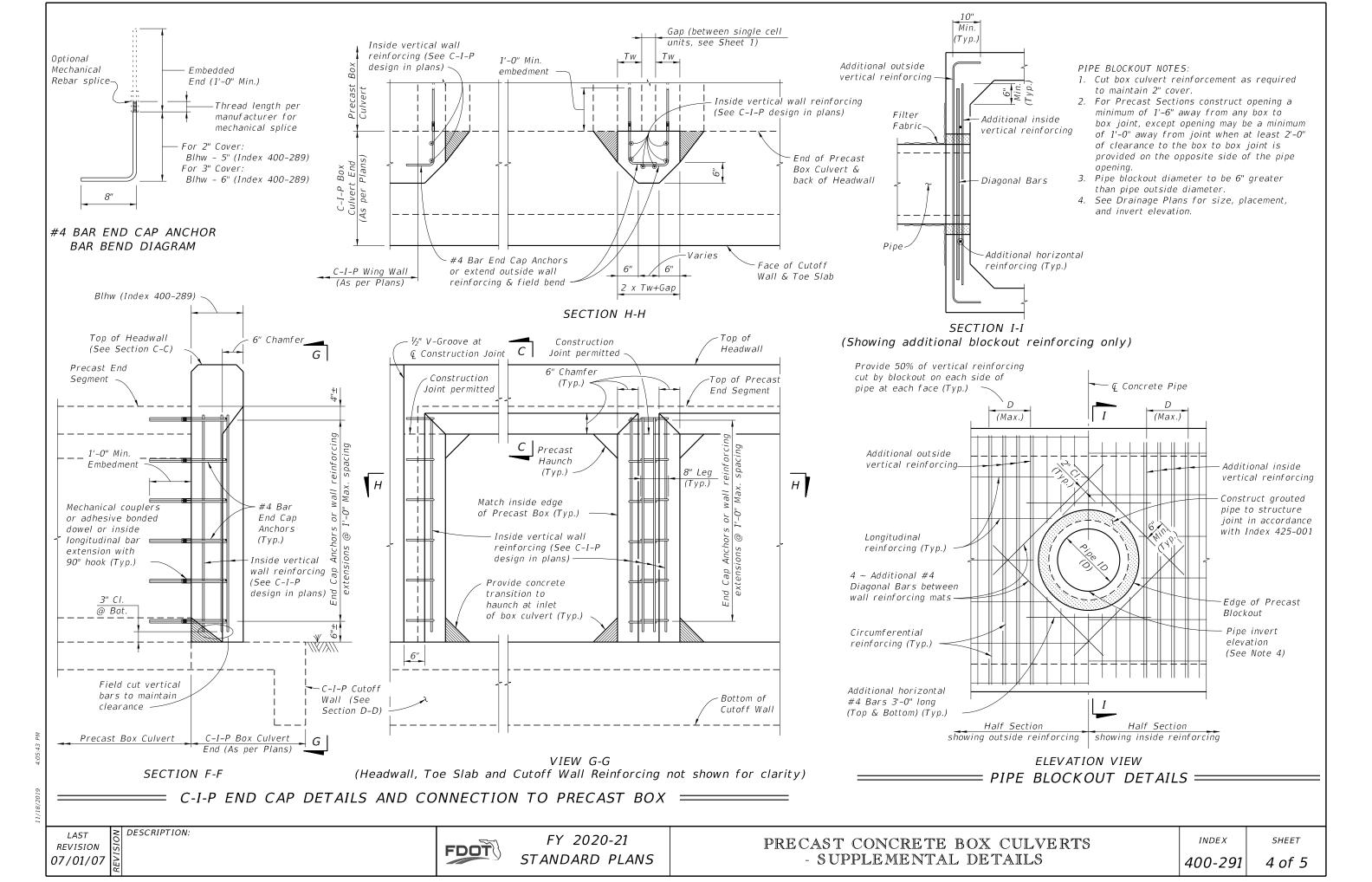
FDOT

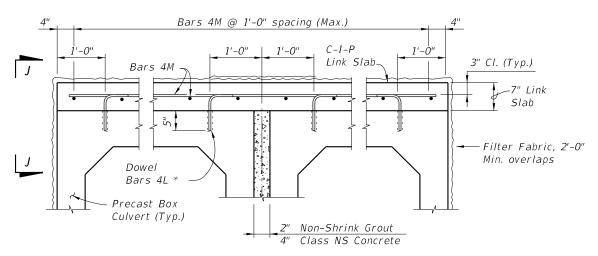
FY 2020-21 STANDARD PLANS PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

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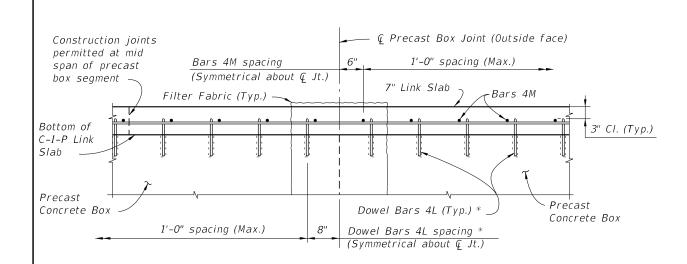






LINK SLAB TYPICAL SECTION (Multiple Barrel Culvert shown, Single Barrel Culvert similar)

* Install dowels with an Adhesive Bonding Material System in accordance with Specification Section 416. The Contractor may substitute mechanical couplers in lieu of adhesive bonded dowels. Shift dowels to clear box culvert reinforcing.



VIEW J-J

LINK SLAB NOTES:

1. Provide a Cast-In-Place Link Slab to ensure uniform joint opening of precast box culverts when the differential settlement shown in the plans exceeds the following limits, except that a Link Slab is not required for differential settlements less than 1/2".

$$\Delta Y \leq \frac{(L)^2}{760 \times R \times W}$$

Where:

 $\Delta Y = Maximum Long-Term Differential Settlement (ft.)$

R = Exterior height of Box Culvert (ft.)

W = Length of Box Culvert Segments (ft.)

L = Effective length for single curvature deflection (ft.)

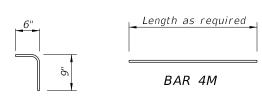
2. Extend Link Slab to back face of headwalls and to limits of existing box culverts for extensions.

ESTIMATED LINK SLA	B QUANTI	TIES
ITEM	UNIT	QUANTITY
Class II or IV Concrete (Culvert)	CY/SF	0.0216
Reinforcing Steel (Roadway)	Lb./SF	1.52

NOTE: Estimated quantities are based the plan area of precast box slabs, and are provided for information only. No additional payment will be made for Link Slabs where these are required for the precast box culverts.

	BILL OF RE	INFORCING STEE	<u>L</u>
MARK	SIZE	NO. REQ'D	LENGTH
L	4	2 per Barrel/Ft.	1'-3"
М	4	As Reqd.	As Reqd.

REINFORCING STEEL BENDING DIAGRAMS



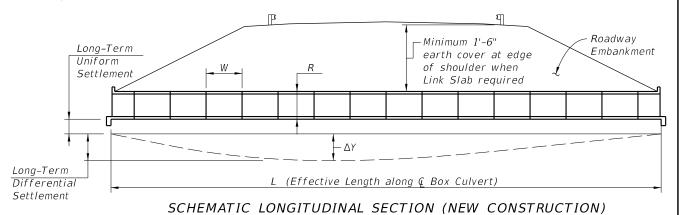
DOWEL BARS 4L

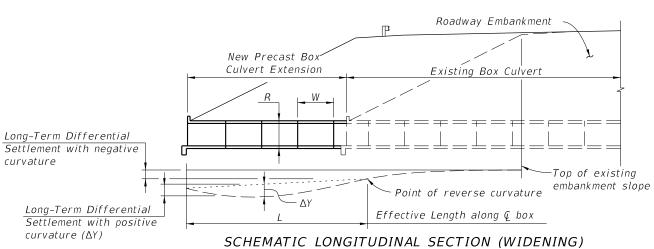
NOTES:

- 1. All bar dimensions are out to out.
- 2. Lap splice length for Bars 4M is 1'-4" minimum.

DESIGN NOTE:

1. Link Slab required when joint openings from differential settlement exceed 1/8" as determined in Link Slab Note 1.





= DIFFERENTIAL SETTLEMENT COUNTERMEASURES FOR PRECAST BOX CULVERTS =

REVISION 01/01/09

DESCRIPTION:

FDOT

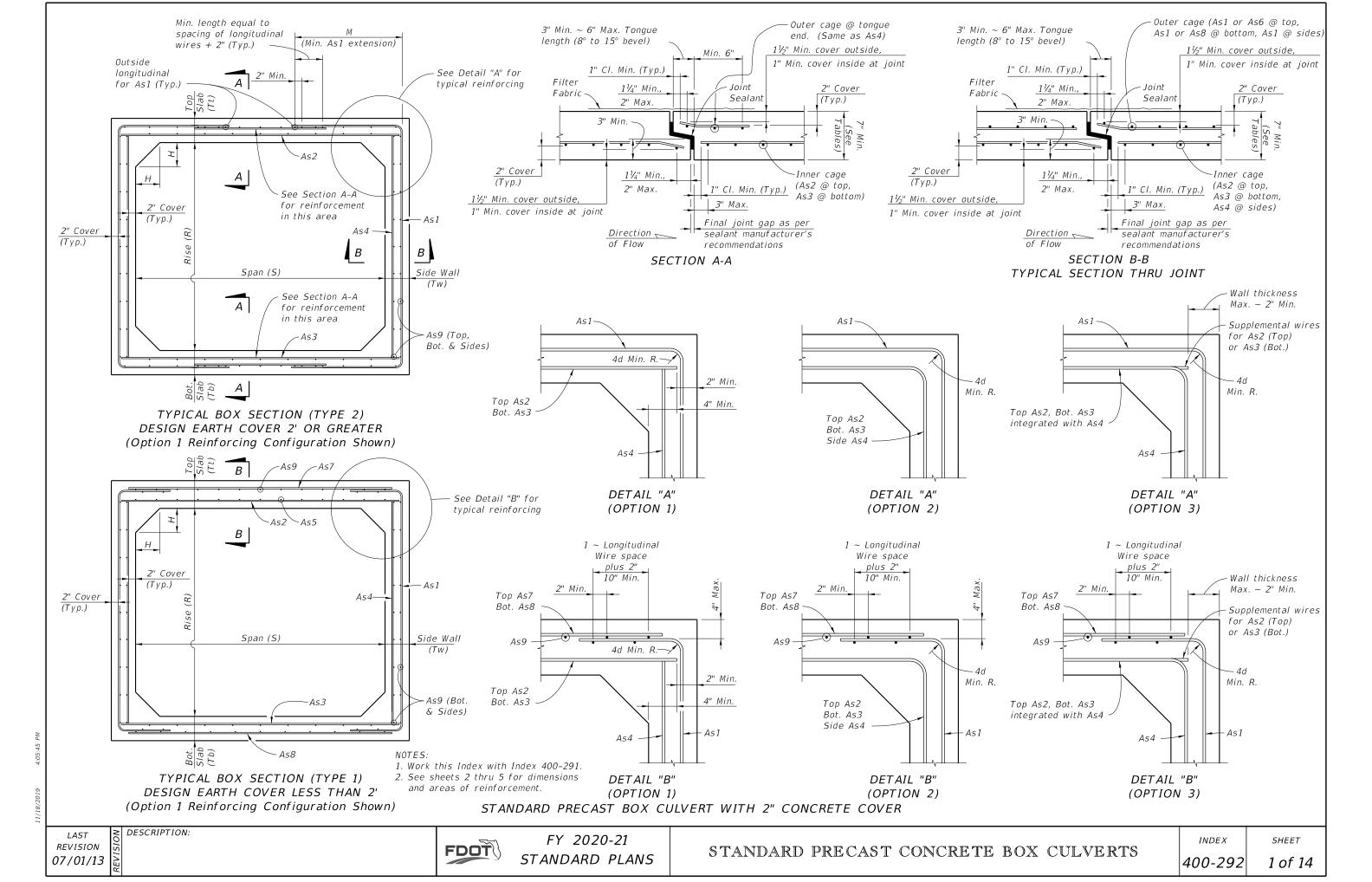
FY 2020-21 STANDARD PLANS

PRECAST CONCRETE BOX CULVERTS - SUPPLEMENTAL DETAILS

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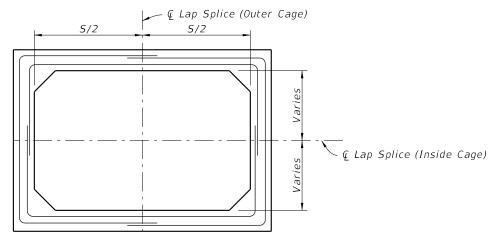


GENERAL NOTES:

- 1. These precast designs may be substituted for cast-in-place box culverts designed to AASHTO LRFD Bridge Design Specifications, 4th Edition. Designs are based on the design criteria shown in FDOT Structures Design Guidelines.
- 2. Loading: HL-93 & any fill heights between the minimum & maximum shown.
- 3. Only one design of precast box culvert is to be used for any installation.
- 4. Reinforcing steel must consist of smooth or deformed welded wire reinforcement (WWR) meeting the requirements of Specification Section 931. Longitudinal reinforcement may consist of reinforcing bars meeting the requirements of Specification Section 931. Minimum cover must be 2" for slightly or moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. The spacing of circumferential wires must not be less than 2" nor more than 4". The spacing of longitudinal wires or bars must not be more than 8".
- 5. As9 longitudinal wires must have a minimum cross-sectional area of 40% of the circumferential wires, but not less than a W2.5 or D4.0 for WWR, or #3 bars for deformed bars.
- 6. Welding of reinforcement must be limited to the locations shown in ASTM C1577 and in accordance with ANSI/AWS D1.4 "Structural Welding Code - Reinforcing Steel".
- 7. For alternate reinforcing configuration Options 2 and 3 shown in Detail "A" and "B" (Sheet 1), As1 may be extended to the middle of either slab and lap spliced with As7 and As8. As4 may be lap spliced at any location or connected to As2 or As3 at corners by welding.
- 8. Haunch dimensions may vary between the minimum and maximum dimensions shown in the Design Tables but only one haunch dimension must be used within the full length of the box culvert installation.

TABLE	1A	STANI	DARD	PRECA	ST BOX CU	LVERT	T DES	SIGNS	(2" (COVE	R) - 3	′&	4' SP.	ANS
SPAN x RISE (S) (R)	TOP	BOT.	SIDE	HAUNCH	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		15		As1 EXT. LENGTH (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.17	0.29	0.21	0.17	0.17	0.17	0.17		-
				4	2' - <3'	0.13	0.28	0.21	0.09	-	-	-		31
					3' - <5'	0.09	0.17	0.17	0.09	-	-	-		31
					5' - 10'	0.09	0.17	0.17	0.09	-	-	-		31
3' x 3'	7	7	7	to	15'	0.09	0.17	0.17	0.09	-	-	-		31
					20'	0.12	0.17	0.17	0.09	-	-	-		31
					25'	0.14	0.18	0.18	0.09	-	-	-		31
				8	30'	0.17	0.21	0.22	0.09	-	-	-		31
					35'	0.19	0.25	0.25	0.09	-	-	-		31
					0.33' - <2'	0.19	0.38	0.26	0.17	0.19	0.17	0.19	e 5	-
				4	2' - <3'	0.19	0.38	0.26	0.09	-	-	-	Note	38
				4	3' - <5'	0.14	0.20	0.22	0.09	-	-	-		38
4' x 3'	7	7	7	to	5' - 10'	0.11	0.17	0.17	0.09	-	-	-	General	38
4 1 2	,	′	′	10	15'	0.15	0.17	0.18	0.09	-	-	-	ene	38
				8	20'	0.20	0.23	0.23	0.09	-	-	-		38
					25'	0.24	0.28	0.29	0.09	-	-	-	See	38
					30'	0.29	0.34	0.35	0.09	-	-	-		38
					0.33' - <2'	0.19	0.41	0.28	0.17	0.21	0.17	0.19		_
				4	2' - <3'	0.19	0.41	0.28	0.09	-	-	-		38
				"	3' - <5'	0.14	0.21	0.24	0.09	-	-	-		38
4' x 4'	7	7	7	to	5' - 10'	0.12	0.17	0.17	0.09	-	-	-		38
4 1 4	/	′	'	10	15'	0.16	0.19	0.20	0.09	-	-	-		38
				8	20'	0.21	0.25	0.25	0.09	-	-	-		38
					25'	0.26	0.31	0.32	0.09	-	-	-		38
					30'	0.31	0.37	0.38	0.09	-	_	-		38

- 9. Submittal of redesign calculations are not required for any increase to the slab and/or wall thickness when the minimum reinforcement areas shown in the Design Tables are provided.
- 10. For Design Earth Cover greater than 10 feet, the Contractor may interpolate the required areas of reinforcement and slab or wall thickness. Interpolated areas of reinforcement, slab or wall thickness must be approved by the Engineer.
- 11. Minimum length of precast box segments is 4 feet and maximum length is 16 feet.
- 12. See Index 400-291 for connections to wingwalls, headwalls and other general details.



SCHEMATIC OF LAP SPLICE LOCATIONS FOR OPTION 2 & 3 REINFORCING CONFIGURATIONS

TABL	.E 1B	- STA	ANDAI	RD PRE	CAST BOX	CULVE	ERT D	ESIG	NS (2	" COV	/ER) -	- 3'	& 4' .	SPANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR			15		As1 EXT.
(S) (R)	TOP	BOT.	I	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M)
(FL.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.20	0.26	0.32	0.20	0.20	0.20	0.20		
				4	2' - <3'	0.16	0.25	0.31	0.10	-	-	-		31
					3' - <5'	0.10	0.20	0.20	0.10	-	-	-		31
					5' - 10'	0.10	0.20	0.20	0.10	-	-	-		31
3' x 3'	8	8	8	to	15'	0.10	0.20	0.20	0.10	-	-	-		31
					20'	0.10	0.20	0.20	0.10	-	-	-		31
					25'	0.11	0.20	0.20	0.10	-	-	-		31
				8	30'	0.13	0.20	0.20	0.10	-	-	-		31
					35'	0.15	0.21	0.21	0.10	-	-	-	2	31
					0.33' - <2'	0.20	0.31	0.22	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.12	0.31	0.22	0.10	-	-	-	Note	38
				,	3' - <5'	0.12	0.20	0.20	0.10	-	-	-	- E	38
4' x 3'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	General	38
	-				15'	0.12	0.20	0.20	0.10	-	-	-	ien	38
				8	20'	0.16	0.20	0.20	0.10	-	-	-		38
				_	25'	0.19	0.24	0.24	0.10	-	-	-	See	38
					30'	0.22	0.28	0.29	0.10	-	-	-		38
					0.33' - <2'	0.20	0.33	0.24	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.17	0.33	0.24	0.10	-	-	-		38
				,	3' - <5'	0.12	0.20	0.20	0.10	-	-	-		38
4' x 4'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-		38
' '					15'	0.13	0.20	0.20	0.10	-	-	-		38
				8	20'	0.16	0.21	0.22	0.10	-	-	-		38
					25'	0.20	0.26	0.27	0.10	-	-	-		38
					30'	0.23	0.31	0.32	0.10	-	-	-		38

NOTES: 1. See Sheet 1 for Reinforcing Details and dimension locations.

2. See Sheet 14 for WWR Bending Diagram.

					CAST BOX	CULV	LNI L						α υ	SPANS
PAN x RISE		/ WAL			DESIGN			R		RCEMEN		iS		AS1 EX
5) (R)	TOP	BOT.		HAUNCH	EARTH COVER ABOVE				(5	q. in./F	τ.)			LENGT (M)
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	TOP SLAB									(in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)	TOF SLAD	As1	As2	As3	As4	As5	As7	As8	A59	(111.)
					0.33' - <2'	0.31	0.48	0.42	0.17	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.48	0.42	0.09	_	-	_]	45
				,	3' - <5'	0.20	0.27	0.27	0.09	-	_	_	1	36
5' x 3'	7	7	7	t 0	5' - 10'	0.17	0.19	0.21	0.09	_	_	_	1	36
<i>J</i> , <i>J</i>	′	'	/	to	15'	0.24	0.25	0.25	0.09	_				35
										_	-	_	-	
				8	20'	0.32	0.33	0.33	0.09	-	-	_		35
					25'	0.39	0.41	0.42	0.09	-	-	_		35
					30'	0.47	0.50	0.50	0.09	-	-	-		35
					0.33' - <2'	0.30	0.51	0.45	0.17	0.23	0.21	0.30		-
				4	2' - <3'	0.30	0.51	0.45	0.09	-	_	_		45
				,	3' - <5'	0.18	0.30	0.29	0.09	_		_	1	45
5' x 4'	7	7	7	4 -	5' - 10'	0.17	0.21	0.23	0.09	_			1	36
J X 4	′	'	/	to	15'								-	
						0.24	0.27	0.28	0.09	-	-	-	_	35
				8	20'	0.31	0.36	0.37	0.09	-	-	-		35
					25'	0.39	0.45	0.46	0.09	-	-	-]	35
					30'	0.46	0.55	0.56	0.09	-	-	-		35
					0.33' - <2'	0.30	0.53	0.48	0.17	0.24	0.21	0.30	1	_
				4	2' - <3'	0.29	0.53	0.48	0.09	-	-	-	1	45
				4	3' - <5'	0.19	0.31	0.31	0.09	_	_	_	1	45
E! E!	7	7	7	l .									-	
5' x 5'	/	/	7	to	5' - 10'	0.19	0.22	0.25	0.09	-	-	-		45
					15'	0.26	0.29	0.31	0.09	-	-	-		36
				8	20'	0.34	0.39	0.40	0.09	-	-	-		35
					25'	0.41	0.49	0.50	0.09	-	-	-		35
					30'	0.49	0.59	0.61	0.09	-	_	_	1	35
	7.5	7	7		0.33' - <2'	0.39	0.54	0.48	0.17	0.22	0.25	0.39	-5	_
	7.3	,	,	4	2' - <3'	0.39	0.58	0.49	0.09	-	-	-	1	43
				4									Note	
	_	_	_		3' - <5'	0.28	0.36	0.36	0.09	-	-	-	<	39
6' x 3'	7	7	7	to	5' - 10'	0.25	0.26	0.28	0.09	-	-	_	General	39
					15'	0.36	0.34	0.34	0.09	-	-	_	ne	38
				12	20'	0.47	0.46	0.46	0.09	-	-	-	<i>Ge</i>	38
	7	7.5	7		25'	0.59	0.57	0.55	0.09	-	_	_		38
	8	8	7		30'	0.60	0.64	0.64	0.09	_	_	_	See	38
	7.5	7	7		0.33' - <2'	0.37	0.58	0.52	0.17	0.24	0.23	0.37		
	1.5												-	12
				4	2' - <3'	0.37	0.61	0.53	0.09	-	-	1-1		43
					3' - <5'	0.26	0.39	0.39	0.09	-	-	_		39
6' x 4'	7	7	7	to	5' - 10'	0.24	0.28	0.31	0.09	-	-	-		39
					15'	0.35	0.37	0.38	0.09	-	-	-		38
				12	20'	0.46	0.50	0.50	0.09	_	-	_		38
	7	7.5	7	1	25'	0.56	0.63	0.60	0.09	_	_	_	1	38
	8	8	7	1	30'	0.58	0.69	0.69	0.09	_	_	_	1	38
	7.5	7	7										1	-
	/.5	/	/	-	0.33' - <2'	0.36	0.60	0.56	0.17	0.25	0.22	0.36	-	
				4	2' - <3'	0.36	0.64	0.56	0.09	-	-	_		43
					3' - <5'	0.26	0.410	0.42	0.09	-	-	-]	43
6' x 5'	7	7	7	to	5' - 10'	0.25	0.30	0.33	0.09	-	-	-		39
					15'	0.34	0.40	0.41	0.09	-	-	-		38
				12	20'	0.46	0.54	0.54	0.09	_	_	_	1	38
	7	7.5	7	12	25'	0.56	0.67	0.65	0.09	_	_	_	1	38
	8	8	8	1	30'	0.60	0.74	0.74		_			1	38
									0.09				-	
	7.5	7	7		0.33' - <2'	0.36	0.63	0.59	0.17	0.26	0.22	.036		
				4	2' - <3'	0.35	0.67	0.59	0.09	-	-	-		52
					3' - <5'	0.27	0.43	0.44	0.09	-	1	-		52
6' x 6'	7	7	7	to	5' - 10'	0.27	0.32	0.35	0.09	-	-	_	1	43
	'	'	•		15'	0.38	0.43	0.44	0.09	_	_	_	1	39
				1.0	20'								1	39
	<u> </u>	7.5	-	12		0.50	0.57	0.59	0.09	-	-	-	-	
	7	7.5	7	-	25'	0.60	0.72	0.70	0.09	-	-	-	-	38
	8	8	7		30'	0.67	0.78	0.79	0.09	-	_	_		38

		- 317	ANDA	KD PKE	CAST BOX	CULVI	ERI L	DESIG	NS (2	" CO	VER)	- 5'	& 6'	<i>SPANS</i>
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EX7
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(F+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
l					0.33' - <2'	0.26	0.39	0.36	0.20	0.20	0.20	0.26		_
				4	2' - <3'	0.26	0.39	0.36	0.10	-	-	-		45
					3' - <5'	0.16	0.23	0.24	0.10	-	-	-		36
5' x 3'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	-	-	-		36
					15'	0.19	0.21	0.22	0.10	-	-	-		35
				8	20'	0.24	0.28	0.28	0.10	-	-	-		35
					25'	0.30	0.34	0.35	0.10	-	-	-		35
					30'	0.36	0.41	0.41	0.10	_	_	_		35
					0.33' - <2'	0.25	0.42	0.38	0.20	0.20	0.20	0.25		_
				4	2' - <3'	0.25	0.42	0.38	0.10	_	_	_		45
				4	3' - <5'	0.16	0.25	0.25	0.10	_	_	_		45
5' x 4'	8	8	8	to	5' - 10'	0.13	0.20	0.20	0.10	_	_	_		36
<i>3</i>		0		10	15'	0.19	0.23	0.24	0.10	_	_	_		35
					20'	0.13	0.30	0.24	0.10	_	_	_		35
				8	25'	0.24	0.37	0.31	0.10					35
					30'	0.35	0.37	0.38	0.10	-	=	-		35
										- 0.20	- 0.20	- 0.25		
					0.33' - <2'	0.25	0.44	0.41	0.20	0.20	0.20	0.25		-
				4	2' - <3'	0.25	0.44	0.41	0.10	-	-	-		45
E, E.					3' - <5'	0.16	0.26	0.27	0.10	-	-	-		45
5' x 5'	8	8	8	to	5' - 10'	0.15	0.20	0.22	0.10	-	-	-		45
					15'	0.20	0.25	0.26	0.10	-	-	-		36
				8	20'	0.26	0.32	0.33	0.10	-	-	-		35
ĺ					25'	0.32	0.40	0.41	0.10	-	-	-		35
					30'	0.37	0.48	0.49	0.10	-	-	-		35
					0.33' - <2'	0.32	0.47	0.41	0.20	0.20	0.25	0.32	5	_
				4	2' - <3'	0.32	0.47	0.41	0.10	-	-	-	Note	43
ĺ					3' - <5'	0.23	0.30	0.31	0.10	-	-	-	N	39
6' x 3'	8	8	8	to	5' - 10'	0.19	0.22	0.24	0.10	-	-	-	General	39
					15'	0.28	0.29	0.29	0.10	-	-	-	ne.	38
				12	20'	0.36	0.38	0.38	0.10	-	-	-	Ge.	38
					25'	0.45	0.47	0.47	0.10	-	-	-	See	38
					30'	0.54	0.57	0.57	0.10	-	-	-	Sé	38
					0.33' - <2'	0.31	0.50	0.44	0.20	0.21	0.23	0.31		_
				4	2' - <3'	0.31	0.50	0.44	0.10	_	_	_		43
				,	3' - <5'	0.23	0.32	0.34	0.10	_	_	_		39
6' x 4'	8	8	8	to	5' - 10'	0.19	0.24	0.26	0.10	-	-	-		39
					15'	0.27	0.31	0.32	0.10	_	_	_		38
				12	20'	0.35	0.41	0.32	0.10	_	_	_		38
				12	25'	0.33	0.41	0.41	0.10	_		_		38
					30'	0.43	0.62	0.62	0.10	_	_	_		38
					0.33' - <2'	0.32	0.52							-
								0.47	0.20	0.22	0.22	0.30		
				4	2' - <3'	0.30	0.52	0.47	0.10	-	-	-		43
6' F'	0	0	,		3' - <5'	0.22	0.34	0.36	0.10	-	_	-		
6' x 5'	8	8	8	to	5' - 10'	0.20	0.26	0.28	0.10	-	-	-		39
					15'	0.27	0.33	0.34	0.10	-	-	-		38
				12	20'	0.36	0.44	0.45	0.10	-	-	-		38
					25'	0.44	0.55	0.55	0.10	-	-	-		38
					30'	0.52	0.66	0.67	0.10	-	-	-		38
					0.33' - <2'	0.30	0.54	0.50	0.20	0.22	0.22	0.30		-
				4	2' - <3'	0.30	0.54	0.50	0.10	-	-	-		52
					3' - <5'	0.23	0.36	0.38	0.10	-	-	-		52
6' x 6'	8	8	8	to	5' - 10'	0.21	0.27	0.30	0.10	-	-	-		43
					15'	0.29	0.35	0.37	0.10	-	-	-		39
				12	20'	0.38	0.47	0.48	0.10	-	-	-		39
					25'	0.47	0.59	0.60	0.10	-	-	-		38
					30'	0.55	0.70	0.71	0.10	-	_	-		38

LAST REVISION 07/01/13

≥ DESCRIPTION:

T	ABLE	3 - 5	STANI	DARD P	RECAST BO	X CU	LVERT	DES	<i>IGNS</i>	(2" (COVER	R) - 7	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	15		As1 EXT
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.37	0.58	0.49	0.20	0.22	0.29	0.37		-
				4	2' - <3'	0.37	0.58	0.49	0.10	-	-	-		43
				t o	3' - <5'	0.30	0.40	0.42	0.10	-	-	-		43
7' x 4'	8	8	8	to	5' - 10'	0.26	0.30	0.33	0.10	-	-	-		43
				12	15'	0.37	0.40	0.40	0.10	-	-	-		41
				12	20'	0.49	0.53	0.53	0.10	-	-	-		41
	8	8	8	7 to	25'	0.60	0.67	0.66	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.68	0.79	0.78	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.60	0.53	0.20	0.23	0.28	0.36		-
				4	2' - <3'	0.36	0.60	0.53	0.10	-	-	-		47
				to	3' - <5'	0.30	0.42	0.45	0.10	-	-	-		43
7' x 5'	8	8	8	to	5' - 10'	0.26	0.32	0.35	0.10	-	-	-		43
				12	15'	0.37	0.43	0.44	0.10	-	-	-	5	41
				12	20'	0.48	0.57	0.57	0.10	-	-	-	te	41
	8	8	8	7 to	25'	0.60	0.72	0.72	0.10	-	-	-	No	41
	8.5	8.5	8	12	30'	0.67	0.84	0.84	0.10	-	-	-	General Note	41
				4	0.33' - <2'	0.36	0.63	0.56	0.20	0.24	0.27	0.36	neı	-
				4	2' - <3'	0.36	0.63	0.56	0.10	-	-	-	е9	59
				to	3' - <5'	0.29	0.44	0.47	0.10	-	-	-	See	47
7' x 6'	8	8	8	10	5' - 10'	0.27	0.34	0.37	0.10	-	-	-	S	43
				12	15'	0.38	0.46	0.46	0.10	-	-	-		41
				12	20'	0.49	0.60	0.61	0.10	-	-	-		41
	8	8	8	7 to	25'	0.61	0.76	0.76	0.10	-	-	-		41
	8.5	8.5	8	12	30'	0.69	0.89	0.89	0.10	-	-	-		41
				4	0.33' - <2'	0.36	0.65	0.58	0.20	0.25	0.27	0.36		-
				4	2' - <3'	0.36	0.65	0.58	0.10	-	-	-		59
				t 0	3' - <5'	0.30	0.46	0.50	0.10	-	-	-		59
7' x 7'	8	8	8	to	5' - 10'	0.30	0.35	0.50	0.10	-	-	-		47
				12	15'	0.41	0.48	0.50	0.10	-	-	-		43
				12	20'	0.53	0.64	0.65	0.10	-	-	-		43
	8	8	8	7 to	25'	0.65	0.80	0.81	0.10	-	-	-		43
	8.5	9	8	12	30'	0.72	0.92	0.91	0.10	-	-	-		41

T	ABLE	4 - 5	STANI	DARD P	RECAST BO	X CU	LV ER 7	DES	IGNS	(2" (COVER	R) - 8	' SPA	NS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R			T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			T	T		T	T		(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	A59	(111.)
	9	8.5	8	4	0.33' - <2'	0.40	0.60	0.52	0.20	0.22	0.28	0.39		-
					2' - <3'	0.45	0.66	0.54	0.10	-	-	-		50
	_			to	3' - <5'	0.39	0.48	0.50	0.10	-	-	-		50
8' x 4'	8	8	8		5' - 10'	0.34	0.38	0.40	0.10	-	-	-		45
				12	15'	0.49	0.51	0.50	0.10	-	-	-		41
					20'	0.65	0.68	0.66	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.76	0.83	0.80	0.10	_	_	-		41
	9.5	9.5	8	12	30'	0.79	0.94	0.92	0.10	-	-	-		41
	9	8.5	8	4	0.33' - <2'	0.38	0.65	0.59	0.20	0.22	0.30	0.37		-
					2' - <3'	0.43	0.69	0.58	0.10	-	-	-		50
0, 5,	0			to	3' - <5'	0.37	0.51	0.53	0.10	_	-	-		45
8' x 5'	8	8	8		5' - 10'	0.33	0.41	0.42	0.10	-	-	-		45
				12	15'	0.48	0.54	0.53	0.10	-	-	-		41
	0.5	0.5		0 1	20'	0.63	0.73	0.70	0.10	-	-	-		41
	8.5	8.5	8	8 to	25'	0.74	0.88	0.86	0.10	-	-	-		41
	9.5	9.5	8	12	30'	0.77	1.00	0.98	0.10	-		- 0.71		41
	9	9	8	4	0.33' - <2'	0.32	0.65	0.58	0.20	0.23	0.25	0.31	e 5	-
					2' - <3'	0.42	0.71	0.61	0.10	-	-	-	Note	50
8' x 6'	8	8	8	to	3' - <5' 5' - 10'	0.37	0.54	0.56	0.10	-	-	-	< -	50 45
8	8	8	8			0.34	0.43	0.45	0.10	-	-	-	General	
				12	15' 20'	0.49	0.37	0.76	0.10	-	-	-	ene	41
	8.5	8.5	8	8 to	25'	0.74	0.77	0.70	0.10		_	_	9 6	41
•	9.5	9.5	8	12	30'	0.74	1.05	1.04	0.10		_	_	See	41
	9.5	9.5	8	12	0.33' - <2'	0.78	0.67	0.60	0.10	0.24	0.24	0.31	- '	-
	9	9	0	4	2' - <3'	0.31	0.74	0.64	0.20	-	0.24	0.51		55
					3' - <5'	0.42	0.74	0.59	0.10		_	_		55
8' x 7'	8	8	8	to	5' - 10'	0.36	0.45	0.33	0.10		_	_		50
	U				15'	0.51	0.43	0.47	0.10	_	_	_		45
				12	20'	0.66	0.81	0.80	0.10		_	_		41
	8.5	8.5	8	8 to	25'	0.78	0.98	0.97	0.10	_	_	_		41
}	9.5	9.5	8	12	30'	0.84	1.10	1.09	0.10	_	_	_		41
	9	9	8		0.33' - <2'	0.32	0.68	0.62	0.20	0.24	0.25	0.32		
				4	2' - <3'	0.32	0.76	0.67	0.20	-	0.23	-		65
					3' - <5'	0.38	0.58	0.61	0.14	_	_	_		65
8' x 8'	8	8	8	to	5' - 10'	0.39	0.46	0.50	0.13	_	_	_		55
					15'	0.55	0.64	0.65	0.10	_	_	_		45
				12	20'	0.71	0.86	0.85	0.10	_	_	_		45
	8.5	8.5	8	8 to	25'	0.84	1.03	1.02	0.10	_	_	_		41
	9.5	9.5	8	12	30'	0.93	1.15	1.15	0.10	_	_	-		41
				1	l .						1			1

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- See Sheet 2 for General Notes.
 See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

PAN x RISE		3 / WAL	L THIC	KNESS	DESIGN			R	EINFOF	RCEMEN	T AREA	15		As1 EX7
(S) (R)	1 ' ' '	BOT.	_	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB				1		1			(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	A58	As9	(111.7
	9.5	9.5	9	4	0.33' - <2'	0.41	0.62	0.53	0.22	0.23	0.34	0.38		-
					2' - <3'	0.44	0.65	0.54	0.11	=	-	-		54
0, 5,				to	3' - <5'	0.39	0.53	0.51	0.11	-	-	-		49
9' x 5'	9	9	9		5' - 10'	0.35	0.42	0.44	0.11	-	-	-		49
				12	15'	0.50	0.56	0.55	0.11	-	_	-		44
	0.5	0.5		0.4.	20'	0.65	0.75	0.73	0.11	_	_	-		44
	9.5	9.5	9	8 to	25'	0.77	0.92	0.90	0.11	-	-	-		44
	10.5	11	9	12	30'	0.81	1.05	1.02	0.11	-	-	-		44
	9.5	9.5	9	4	0.33' - <2'	0.38	0.64	0.56	0.23	0.23	0.33	0.37		-
					2' - <3'	0.43	0.67	0.57	0.11	_	-	-		54
01 01		9	9	to	3' - <5'	0.37	0.55	0.54	0.11	-	-	-		49
9' x 6'	9	9	9		5' - 10'	0.35	0.45	0.47	0.11	-	-	-		49
				12	15'	0.49	0.60	0.59	0.11	-	-	-		44
	9.5	9.5	9	0 +0	20' 25'	0.65	0.80	0.78	0.11	-	-	-		44
	10.5	9.5	9	8 to 12	30'	0.76 0.80	1.10	1.08	0.11	-	-	_		44
	9.5		9	12	0.33' - <2'								10	<u> </u>
	9.5	9.5	9	4	2' - <3'	0.37	0.67	0.59	0.22	0.23	0.32	0.37	е 5	59
					3' - <5'	0.42	0.58	0.56	0.11	_	_	_	Note	54
9' x 7'	9	9	9	to	5' - 10'	0.36	0.38	0.30	0.11	_	_	_		49
9 x /	9	9	9		15'	0.50	0.47	0.49	0.11	_	_	_	era	49
				12	20'	0.66	0.84	0.80	0.11	_	_	_	General	44
	9.5	9.5	9	8 to	25'	0.77	1.02	1.00	0.11	_	_	_	6	44
	10.5	11	9	12	30'	0.81	1.15	1.13	0.11	_	_	_	See	44
	9.5	9.5	9		0.33' - <2'	0.37	0.68	0.61	0.22	0.23	0.31	0.37		_
	3.5	3.5		4	2' - <3'	0.42	0.71	0.62	0.11	-	-	-		59
					3' - <5'	0.37	0.60	0.59	0.11	_	_	_		59
9' x 8'	9	9	9	to	5' - 10'	0.38	0.49	0.51	0.11	_	_	_		54
3 X G					15'	0.53	0.66	0.66	0.11	_	_	_		44
				12	20'	0.68	0.88	0.87	0.11	_	_	_		44
	9.5	9.5	9	8 to	25'	0.81	1.07	1.05	0.11	_	_	_		44
	10.5	11	9	12	30'	0.86	1.20	1.18	0.11	-	_	-		44
	9.5	9.5	9		0.33' - <2'	0.38	0.70	0.63	0.22	0.23	0.32	0.38		_
	1.2	1		4	2' - <3'	0.43	0.73	0.65	0.15	-	-	-		72
					3' - <5'	0.38	0.62	0.61	0.15	_	_	_		72
9' x 9'	9	9	9	to	5' - 10'	0.41	0.50	0.53	0.14	-	_	-		59
-		-		1.0	15'	0.57	0.69	0.70	0.12	-	-	-		49
				12	20'	0.73	0.92	0.91	0.11	-	-	-		49
	9.5	10	9	8 to	25'	0.83	1.11	1.09	0.11	_	_	-		44
	10.5	11	9	12	30'	0.93	1.25	1.23	0.11	_	_	_		44

TABL	E 6 -	STAI	VDARI) PREC	AST BOX C	ULVEI	RT DE	SIGN	5 (2"	COVI	ER) -	10' SI	PANS	
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R		RCEMEN		15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	ABOV E									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.46	0.62	0.52	0.24	0.24	0.41	0.45		-
				4	2' - <3'	0.46	0.62	0.52	0.12	_	_	_		58
					3' - <5'	0.42	0.54	0.50	0.12	_	_	_		53
10' x 5'	10	10	10	to	5' - 10'	0.38	0.46	0.49	0.12	_	_	_		52
10 / 3					15'	0.52	0.59	0.58	0.12	_	_	_		47
				12	20'	0.69	0.78	0.76	0.12	_	_	_		47
	10.5	10.5	10	8 to	25 [']	0.81	0.97	0.93	0.12	_	_	_		47
	11.5	12	10	12	30'	0.87	1.11	1.11	0.12	_	_	_		47
	11.5	12	10	12	0.33' - <2'	0.44	0.64	0.54	0.12		0.39	0.44		
				4						0.24	0.39	0.44		-
					2' - <3'	0.44	0.64	0.54	0.12	_	-	-		58
101 61	1.0	1.0	1.0	to	3' - <5'	0.39	0.57	0.52	0.12	-	_	-		52
10' x 6'	10	10	10		5' - 10'	0.37	0.48	0.52	0.12	-	-	-		52
				12	15'	0.51	0.62	0.61	0.12	-	-	-		47
	10 -	4.5 =			20'	0.67	0.83	0.80	0.12	-	-	-		47
	10.5	10.5	10	8 to	25'	0.79	1.02	0.99	0.12	-	-	-		47
	11.5	12	10	12	30'	0.85	1.17	1.14	0.12	-	-	-		47
				4	0.33' - <2'	0.43	0.66	0.57	0.24	0.24	0.38	0.43		-
				'	2' - <3'	0.43	0.66	0.57	0.12	-	-	-		58
				to	3' - <5'	0.38	0.59	0.55	0.12	-	-	-		58
10' x 7'	10	10	10	10	5' - 10'	0.37	0.50	0.54	0.12	-	-	-		52
				1 7	15'	0.52	0.66	0.65	0.12	-	_	-	5	47
				12	20'	0.67	0.87	0.85	0.12	_	-	-	te	47
	10.5	10.5	10	8 to	25'	0.79	1.07	1.04	0.12	-	-	-	Note	47
	11.5	12	10	12	30'	0.84	1.22	1.19	0.12	-	-	-	General	47
					0.33' - <2'	0.43	0.68	0.60	0.24	0.24	0.38	0.43	ier	_
				4	2' - <3'	0.43	0.68	0.60	0.12	_	_	_	3er	64
					3' - <5'	0.38	0.62	0.57	0.12	_	_	_		58
10' x 8'	10	10	10	to	5' - 10'	0.38	0.52	0.57	0.12	_	_	_	See	52
10 / 0	10	10	10		15'	0.53	0.69	0.68	0.12	_	_	_		47
				12	20'	0.68	0.03	0.89	0.12	_	_	_		47
	10.5	10.5	10	8 to	25'	0.81	1.12	1.09	0.12	_	- -	_		47
	11.5	10.5	10	12	30'	0.81	1.12	1.09	0.12	_		_		47
	11.5	12	10	12							0.30	0.43		
				4	0.33' - <2'	0.43	0.70	0.62	0.24	0.24	0.38	0.43		7.0
					2' - <3'	0.43	0.70	0.62	0.12	-	_	-		70
4.04				to	3' - <5'	0.39	0.64	0.60	0.12	-	-	-		64
10' x 9'	10	10	10		5' - 10'	0.40	0.54	0.59	0.12	-	-	-		58
				12	15'	0.56	0.72	0.72	0.12	-	-	-		52
					20'	0.71	0.95	0.94	0.12	-	-	-		47
	10.5	11	10	8 to	25'	0.82	1.15	1.13	0.12	-	-	-		47
	11.5	12	10	12	30'	0.90	1.32	1.30	0.12	-	-	-		47
				4	0.33' - <2'	0.44	0.71	0.64	0.24	0.24	0.38	0.44		-
				7	2' - <3'	0.44	0.71	0.64	0.17	-	_	_		79
				l +0	3' - <5'	0.40	0.65	0.62	0.16	-	-	-		70
10' × 10'	10	10	10	to	5' - 10'	0.44	0.56	0.61	0.15	-	-	-		64
				1 2	15'	0.60	0.75	0.76	0.12	-	-	-		52
				12	20'	0.76	0.99	0.99	0.12	-	-	-		52
	10.5	11	10	8 to	25'	0.86	1.20	1.18	0.12	-	_	-		47
	11.5	12	10	12	30'	0.97	1.36	1.35	0.13	-				47

- See Sheet 1 for Reinforcing Details and dimension locations.
 See Sheet 2 for General Notes.
 See Sheet 14 for WWR Bending Diagram.

≥ DESCRIPTION: LAST REVISION 07/01/13



TAB	BLE 7	- STA	ANDAF	RD PRE	CAST BOX	CULVE	RT D	ESIG	NS (2	" COV	/ER) -	- 11' S	PANS	
SPAN x RISE	SLAE	/ WAL	L THIC	KNESS	DESIGN			R	EINFOF	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	вот.	l	HAUNCH					(5	q. in./F	t.)			LENGTH
(5+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.51	0.57	0.47	0.27	0.27	0.45	0.48		-
				,	2' - <3'	0.51	0.57	0.47	0.14	-	-	-		62
				to	3' - <5'	0.48	0.57	0.46	0.14	-	-	-		62
11' × 4'	11	11	11		5' - 10'	0.47	0.50	0.50	0.14	-	-	-		55
				12	15'	0.59	0.58	0.56	0.14	-	-	-		55
					20'	0.77	0.77	0.74	0.14	-	-	-		55
	11.5	11.5	11	8 to	25'	0.92	0.95	0.91	0.14	-	-	-		55
	13	13	11	12	30'	0.94	1.09	1.06	0.14	-	-	-		55
				4	0.33' - <2'	0.45	0.62	0.52	0.27	0.27	0.41	0.45		-
				·	2' - <3'	0.45	0.62	0.52	0.14	-	-	-		62
				to	3' - <5'	0.42	0.58	0.51	0.14	-	-	_		55
11' x 6'	11	11	11		5' - 10'	0.43	0.56	0.56	0.14	-	-	-		55
				12	15'	0.54	0.65	0.64	0.14	-	-	-		50
					20'	0.70	0.86	0.83	0.14	-	-	-		50
	11.5	11.5	11	8 to	25'	0.83	1.07	1.03	0.14	-	-	-		50
	13	13	11	12	30'	0.85	1.22	1.19	0.14	-	-	-		50
				4	0.33' - <2'	0.42	0.67	0.57	0.27	0.27	0.39	0.43	5	-
					2' - <3'	0.43	0.67	0.57	0.14	-	-	-	Note	62
				to	3' - <5'	0.39	0.63	0.56	0.14	-	-	-		62
11' x 8'	11	11	11		5' - 10'	0.43	0.60	0.61	0.14	-	-	-	General	55
				12	15'	0.54	0.72	0.71	0.14	-	-	-	eue	50
				_	20'	0.70	0.94	0.92	0.14	-	-	-		50
	11.5	11.5	11	8 to	25'	0.82	1.16	1.13	0.14	-	-	-	See	50
	13	13	11	12	30'	0.86	1.32	1.30	0.14	-	-	-	S	50
				4	0.33' - <2'	0.44	0.71	0.62	0.27	0.27	0.38	0.44		
					2' - <3'	0.44	0.71	0.62	0.14	-	-	-		75
	١			to	3' - <5'	0.41	0.67	0.61	0.14	-	-	-		69
11' × 10'	11	11	11		5' - 10'	0.47	0.64	0.66	0.14	-	_	-		62
				12	15'	0.59	0.78	0.78	0.14	_	_	_		55
	115	1.2	1.1	0.5-	20'	0.75	1.03	1.01	0.14	-	_	_		50
	11.5	12	11	8 to	25'	0.85	1.24	1.22	0.14	-	-	-		50
	13	13.5	11	12	30'	0.91	1.40	1.39	0.14	- 0.27	-	- 0.45		50
				4	0.33' - <2'	0.45	0.72	0.64	0.27	0.27	0.39	0.45		-
					2' - <3'	0.45	0.72	0.64	0.18	-	_	-		86
111 111	1 1	,,	1 1	to	3' - <5'	0.42	0.69	0.63	0.18	-	_	-		75
11' × 11'	11	11	11		5' - 10' 15'	0.51	0.66	0.69	0.16	-	_	-		69 55
				12		0.63	0.81	0.82	0.14	-	-	_		55
	11.5	12	11	8 to	20' 25'	0.80	1.07 1.29	1.06 1.27	0.14		-			55
	11.5	13.5	11	12	30'	0.91	1.29	1.27	0.14	-	-	-		50
	13	ر.د۱	1 1	12] 30	0.99	1.44	1.44	0.14					טכן

TAB	LE 8	- STA	ANDAF	RD PRE	CAST BOX (CULVE	RT D	ESIGI	V <i>S (2</i>	" COV	'ER) -	12' S	SPANS	5
SPAN x RISE	SLAE	/ WAL	L THIC		DESIGN			R			T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			r						(M)
(FL.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.52	0.57	0.45	0.29	0.29	0.47	0.49		-
					2' - <3'	0.52	0.57	0.45	0.15	-	-	-		73
				to	3' - <5'	0.50	0.54	0.45	0.15	-	-	-		66
12' x 4'	12	12	12		5' - 10'	0.50	0.52	0.52	0.15	-	-	-		66
				12	15'	0.63	0.61	0.59	0.15	-	-	-		59
					20'	0.82	0.81	0.77	0.15	-	-	-		59
	12.5	12.5	12	8 to	25'	0.99	0.99	0.95	0.15	-	-	-		59
	14	14	12	12	30'	1.03	1.15	1.11	0.15	-	-	-		59
				4	0.33' - <2'	0.47	0.62	0.51	0.29	0.29	0.42	0.46		-
				,	2' - <3'	0.47	0.62	0.51	0.15	-	-	-		66
				to	3' - <5'	0.45	0.60	0.51	0.15	-	-	-		59
12' x 6'	12	12	12		5' - 10'	0.47	0.59	0.59	0.15	-	-	-		59
				12	15'	0.57	0.68	0.66	0.15	-	-	-		53
					20'	0.74	0.90	0.86	0.15	-	-	-		53
	12.5	12.5	12	8 to	25'	0.88	1.11	1.06	0.15	-	-	-		53
	14	14.5	12	12	30'	0.92	1.27	1.24	0.15	-	_	-		53
				4	0.33' - <2'	0.44	0.67	0.56	0.29	0.29	0.40	0.44	5	-
				7	2' - <3'	0.44	0.67	0.56	0.15	-	-	-	te	66
				to	3' - <5'	0.41	0.64	0.56	0.15	-	-	-	General Note	59
12' x 8'	12	12	12	10	5' - 10'	0.45	0.63	0.64	0.15	-	-	-	le.	59
				12	15'	0.56	0.75	0.73	0.15	-	-	-	Jer	53
				12	20'	0.72	0.98	0.95	0.15	-	-	-	(<i>ee</i>)	53
	12.5	13	12	8 to	25'	0.85	1.20	1.16	0.15	-	-	-	See	53
	14	14.5	12	12	30'	0.89	1.38	1.35	0.15	-	-	-	Se	53
				4	0.33' - <2'	0.44	0.71	0.60	0.29	0.29	0.39	0.44	1	-
				4	2' - <3'	0.44	0.71	0.60	0.15	-	-	-		73
				to	3' - <5'	0.42	0.68	0.60	0.15	-	-	-		66
12' x 10'	12	12	12	10	5' - 10'	0.47	0.67	0.69	0.15	-	-	-	1	59
				12	15'	0.59	0.81	0.81	0.15	_	_	-		53
				12	20'	0.75	1.06	1.04	0.15	-	-	-		53
	12.5	13	12	8 to	25'	0.87	1.30	1.26	0.15	-	-	-	1	53
	14	14.5	12	12	30'	0.92	1.47	1.45	0.15	_	_	_	1	53
				1	0.33' - <2'	0.46	0.74	0.64	0.29	0.29	0.40	0.46	1	_
				4	2' - <3'	0.46	0.74	0.64	0.20	-	-	-	1	93
					3' - <5'	0.42	0.72	0.64	0.20	_	_	_		80
12' x 12'	12	12	12	to	5' - 10'	0.54	0.71	0.74	0.18	_	_	_		73
				1.2	15'	0.66	0.87	0.89	0.15	_	_	_	1	59
				12	20'	0.83	1.14	1.13	0.15	_	_	_		59
	12.5	13	12	8 to	25'	0.96	1.39	1.37	0.15	_	_	_		53
	14	14.5	12.5	12	30'	1.05	1.56	1.56	0.15	_	_	_		53
			1			1.00	1.50	1.50		l	l	l	I	1 33

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

≥ DESCRIPTION: REVISION 07/01/13



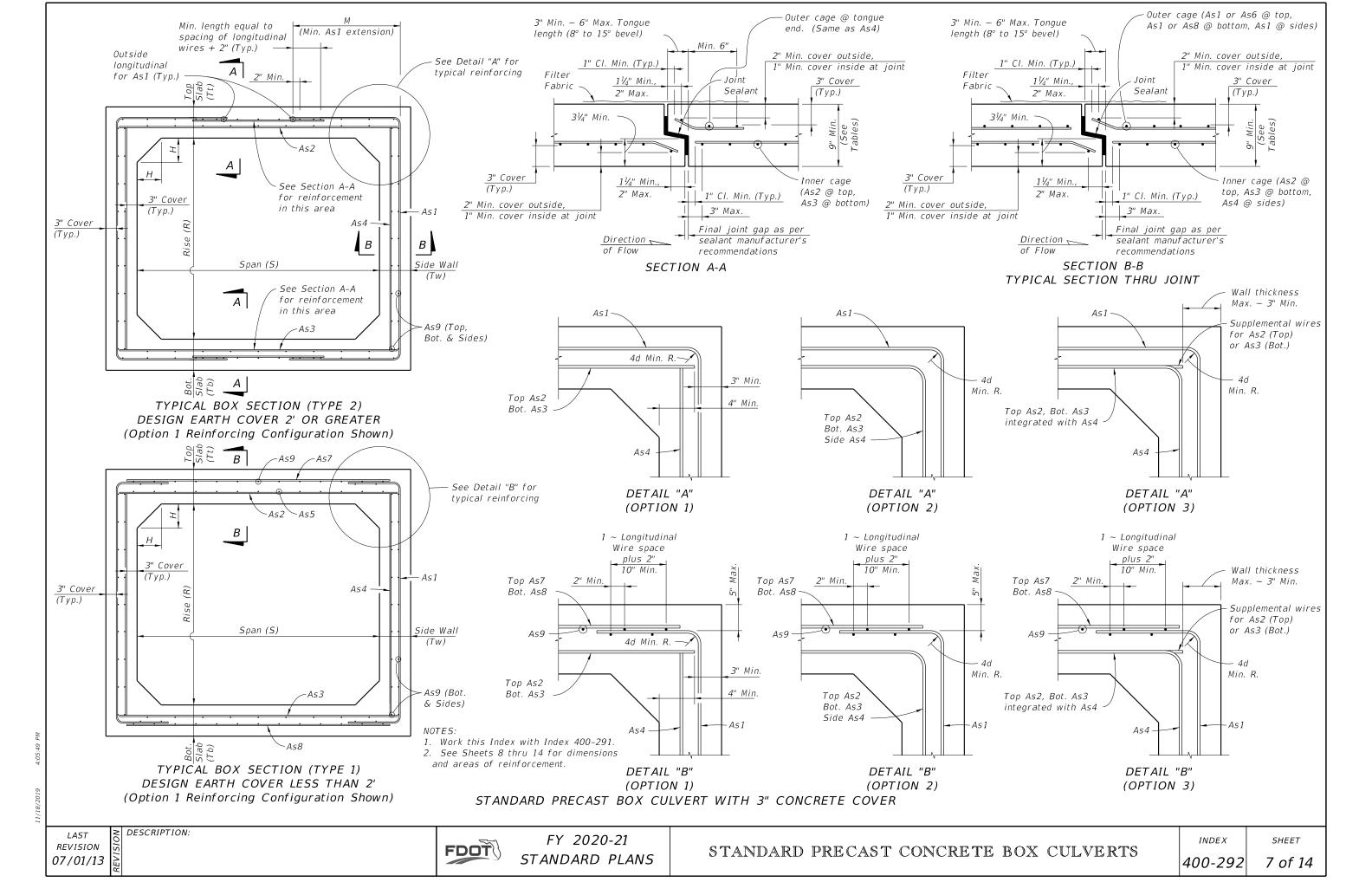


TABLE	9A -	STAN	DARD	PRECA	AST BOX CL	ILVER	T DE.	SIGNS	5 (3"	COVE	R) - 3	3' & 4	l' SPA	ANS
SPAN x RISE (S) (R)	SLAB TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		IS		As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.22	0.24	0.22	0.22	0.22	0.22	0.22		-
					2' - <3'	0.11	0.23	0.22	0.11	-	-	-		31
				4	3' - <5'	0.11	0.22	0.22	0.11	-	-	-		31
					5' - 10'	0.11	0.22	0.22	0.11	-	-	-		31
3' x 3'	9	9	9	to	15'	0.11	0.22	0.22	0.11	-	-	-		31
					20'	0.13	0.22	0.22	0.11	-	-	-		31
				8	25'	0.16	0.22	0.22	0.11	-	-	-		31
					30'	0.19	0.24	0.25	0.11	-	-	-		31
					35'	0.22	0.28	0.29	0.11	ı	-	-		31
					0.33' - <2'	0.22	0.32	0.24	0.22	0.22	0.22	0.22	5	_
				4	2' - <3'	0.17	0.31	0.24	0.11	_	-	-	Note	38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 3'	9	9	9	to	5' - 10'	0.13	0.22	0.22	0.11	-	-	-	General	38
					15'	0.17	0.22	0.22	0.11	-	-	-	neı	38
				8	20'	0.23	0.26	0.27	0.11	-	-	-	Ge	38
					25'	0.28	0.32	0.34	0.11	1	-	-	66	38
					30'	0.33	0.39	0.40	0.11	ı	-	-	S	38
					0.33' - <2'	0.22	0.34	0.26	022	0.22	0.22	0.22		_
				4	2' - <3'	0.17	0.33	0.26	0.11	-	-	-		38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-		38
4' x 4'	9	9	9	to	5' - 10'	0.14	0.22	0.22	0.11	-	-	-		38
					15'	0.19	0.22	0.23	0.11	-	-	-		38
				8	20'	0.24	0.28	0.30	0.11	-	-	-		38
					25'	0.29	0.36	0.37	0.11	-	-	-		38
					30'	0.34	0.43	0.45	0.11	_	-	-		38

TABLE	9B -	STAN	DARD	PRECA	AST BOX CL	ILVER	T DE	SIGNS	5 (3"	COVE	R)	3' & 4	l' SPA	NS
SPAN x RISE	SLAE	3 / WAL		KNESS	DESIGN			R			T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH					(5	q. in./F	t.)			LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.24	0.24	0.24	0.24	0.24	0.24		-
					2' - <3'	0.12	0.24	0.24	0.24	ı	-	-		31
				4	3' - <5'	0.12	0.24	0.24	0.24	ı	-	-		31
					5' - 10'	0.12	0.24	0.24	0.24	ı	-	-		31
3' x 3'	10	10	10	to	15'	0.12	0.24	0.24	0.24	ı	-	-		31
					20'	0.12	0.24	0.24	0.24	-	_	-		31
				8	25'	0.13	0.24	0.24	0.24	-	-	-		31
					30'	0.15	0.24	0.24	0.12	-	-	-		31
					35'	0.18	0.24	0.24	0.12	-	-	-		31
					0.33' - <2'	0.24	0.26	0.24	0.24	0.24	0.24	0.24	5	-
				4	2' - <3'	0.14	0.26	0.24	0.12	1	-	-	Note	38
					3' - <5'	0.12	0.24	0.24	0.12	ı	-	-		38
4' x 3'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	ı	-	-	le,	38
					15'	0.14	0.24	0.24	0.12	-	-	-	General	38
				8	20'	0.18	0.24	0.24	0.12	-	-	-	<i>Ge</i>	38
					25'	0.22	0.26	0.27	0.12	-	-	-	ee	38
					30'	0.26	0.31	0.32	0.12	-	-	-	Sé	38
					0.33' - <2'	0.24	0.28	0.24	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.14	0.28	0.24	0.12	-	_	-		38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-		38
4' x 4'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-		38
					15'	0.15	0.24	0.24	0.12	-	-	-		38
				8	20'	0.19	0.24	0.24	0.12	-	-	-		38
					25'	0.23	0.28	0.30	0.12	-	-	-		38
					30'	0.27	0.34	0.35	0.12	_	_	_		38

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

LAST	NC	DESCRIPTION:
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SHEET 8 of 14

TABLE	10A -	STAN	IDARE	PREC.	AST BOX C	JLVEF	RT DE	SIGN.	5 (3"	COVE	R) -	5' & (5' SP,	ANS
PAN x RISE		/ WAL			DESIGN			R		RCEMEN		15		As1 EX
S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGT
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB			1	Г	1			Г	(M)
(FL.)	(in.)	(in.)	(in.)	(in.)	TUP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.27	0.39	0.37	0.22	0.22	0.22	0.27		_
				4	2' - <3'	0.26	0.39	0.37	0.11	_	-	_		45
					3' - <5'	0.19	0.24	0.25	0.11	_	-	-		36
5' x 3'	9	9	9	to	5' - 10'	0.20	0.22	0.22	0.11	-	-	-		36
					15'	0.28	0.28	0.30	0.11	_	_	_		35
				8	20'	0.37	0.38	0.39	0.11	_	_	_		35
					25'	0.45	0.48	0.49	0.11	_	_	_		35
					30'	0.54	0.58	0.59	0.11	_		_		35
					0.33' - <2'	0.26	0.42	0.39	0.22	0.22	0.22	0.26		_
					2' - <3'	0.26	0.42	0.39	0.22	-	-	-		45
				4										
E. 41					3' - <5'	0.19	0.26	0.27	0.11	-	-	-		45
5' x 4'	9	9	9	to	5' - 10'	0.20	0.22	0.23	0.11	-	-	-		36
					15'	0.27	0.31	0.33	0.11	-	-	-		35
				8	20'	0.36	0.42	0.43	0.11	-	-	-		35
					25'	0.44	0.52	0.54	0.11	-	-	-		35
					30'	0.53	0.63	0.65	0.11	-	-	-		35
					0.33' - <2'	0.27	0.44	0.42	0.22	0.22	0.22	0.27		-
				4	2' - <3'	0.27	0.44	0.42	0.11	-	-	-		45
					3' - <5'	0.20	0.27	0.28	0.11	_	-	_		45
5' x 5'	9	9	9	to	5' - 10'	0.22	0.23	0.26	0.11	-	-	-		45
					15'	0.30	0.34	0.36	0.11	_	_	_		36
				8	20'	0.38	0.45	0.47	0.11	_	_	_		35
					25'	0.47	0.56	0.59	0.11	_	_	_		35
					30'	0.55	0.68	0.71	0.11	_	_	_		35
					0.33' - <2'				0.11		0.25	0.24	10	-
						0.34	0.47	0.42		0.22	0.25	0.34	5 5	- 43
				4	2' - <3'	0.34	0.47	0.42	0.11	-	-	-	ote	43
					3' - <5'	0.27	0.31	0.32	0.11	-	-	-	General Note	39
6' x 3'	9	9	9	to	5' - 10'	0.29	0.26	0.28	0.11	-	-	-	ra	39
					15'	0.42	0.39	0.40	0.11	-	-	-	ine	38
				12	20'	0.55	0.52	0.53	0.11	-	-	-		38
					25'	0.68	0.66	0.67	0.11	-	-	-	See	38
					30'	0.82	0.81	0.82	0.11	-	-	-	S	38
					0.33' - <2'	0.33	0.50	0.46	0.22	0.22	0.23	0.33		_
				4	2' - <3'	0.33	0.50	0.46	0.11	_	-	_		43
					3' - <5'	0.27	0.33	0.35	0.11	_	-	_		39
6' x 4'	9	9	9	to	5' - 10'		0.29	0.31	0.11	-	-	-		39
					15'	0.40	0.43	0.45	0.11	-	-	-		38
				12	20'	0.52	0.57	0.59	0.11	_	_	_		38
				12	25'	0.65	0.73	0.74	0.11	-	-	-		38
					30'	0.78	0.88	0.90	0.11	_	_			38
					0.33' - <2'	0.33	0.52	0.49	0.22	0.22	0.23	0.33		-
					2' - <3'	0.33	0.52	0.49	0.22		0.23	0.55		43
				4						-				
Cl 51					3' - <5'	0.27	0.35	0.37	0.11	-	-	-		43
6' x 5'	9	9	9	to	5' - 10'	0.29	0.31	0.34	0.11	-	-	-		39
					15'	0.41	0.46	0.49	0.11	-	-	-		38
				12	20'	0.53	0.62	0.64	0.11	-	-	-		38
					25'	0.66	0.78	0.80	0.11	-	-	-		38
					30'	0.78	0.95	0.97	0.11	-	-	-		38
					0.33' - <2'	0.34	0.55	0.51	0.22	0.22	0.24	0.34		-
				4	2' - <3'	0.34	0.54	0.51	0.11	-	-			52
				'	3' - <5'	0.29	0.37	0.39	0.11	-	-	-		52
6' x 6'	9	9	9	to	5' - 10'	0.32	0.34	0.37	0.11	_	_	_		43
.				"	15'	0.44	0.50	0.53	0.11	_	_	_		39
				12	20'	0.57	0.66	0.70	0.11	_	_	-		39
				12	25'	0.70	0.84	0.70	0.11	_	_	_		38
					30'	0.70	1.02	1.05	0.11	_	_	_		38
					20	0.03	1.02	1.00	U.11					

IADLE	IUB -	SIAN	IDAKL) PREC.	AST BOX CU	JLVEF	(I DE	SIGN.	5 (3"	COVE	:R) -	5' & 6	5' SP,	ANS
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR			15		As1 EXT
(S) (R)	TOP	BOT.	_	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(E+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
					0.33' - <2'	0.24	0.33	0.32	0.24	0.24	0.24	0.24		_
				4	2' - <3'	0.22	0.33	0.32	0.12	-	-	-		45
					3' - <5'	0.16	0.24	0.24	0.12	-	-	-		36
5' x 3'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-		36
					15'	0.23	0.24	0.24	0.12	-	-	-		35
				12	20'	0.29	0.30	0.31	0.12	-	-	-		35
					25'	0.36	0.38	0.39	0.12	-	-	-		35
					30'	0.43	0.46	0.47	0.12	-	-	-		35
					0.33' - <2'	0.24	0.35	0.34	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.22	0.35	0.34	0.12	-	-	-		45
					3' - <5'	0.15	0.24	0.24	0.12	-	-	-		45
5' x 4'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-		36
					1 <i>5</i> ′	0.22	0.25	0.27	0.12	-	-	-		35
				12	20'	0.29	0.33	0.34	0.12	-	-	-		35
					<i>25</i> ′	0.36	0.41	0.43	0.12	-	-	-		35
					30'	0.42	0.50	0.51	0.12	-	-	-		35
					0.33' - <2'	0.24	0.37	0.36	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.21	0.37	0.36	0.12	-	-	-		45
					3' - <5'	0.16	0.24	0.25	0.12	-	-	-		45
5' x 5'	10	10	10	to	5' - 10'	0.17	0.24	0.24	0.12	-	-	-		45
					15'	0.24	0.27	0.29	0.12	-	-	-		36
				12	20'	0.30	0.36	0.38	0.12	-	-	-		35
					25'	0.37	0.44	0.47	0.12	-	-	-		35
					30'	0.44	0.53	0.56	0.12	-	-	-		35
					0.33' - <2'	0.28	0.40	0.36	0.24	0.24	0.24	0.28	5	-
				4	2' - <3'	0.28	0.40	0.36	0.12	-	-	-	te	43
					3' - <5'	0.22	0.26	0.28	0.12	-	-	-	Note	39
6' x 3'	10	10	10	to	5' - 10'	0.24	0.24	0.24	0.12	-	-	-	al	39
					15'	0.34	0.31	0.32	0.12	-	-	-	rer	38
				12	20'	0.44	0.41	0.42	0.12	-	-	-	General	38
					25'	0.54	0.52	0.53	0.12	-	-	-	See	38
					30'	0.64	0.63	0.64	0.12	-	-	-	Se	38
					0.33' - <2'	0.27	0.42	0.39	0.24	0.24	0.24	0.27		_
				4	2' - <3'	0.27	0.42	0.39	0.12	_	_	-		43
				,	3' - <5'	0.21	0.28	0.30	0.12	_	_	_		39
6' x 4'	10	10	10	to	5' - 10'	0.23	0.24	0.25	0.12	-	-	-		39
					15'	0.32	0.34	0.35	0.12	-	-	-		38
				12	20'	0.42	0.45	0.47	0.12	-	_	-		38
				'-	25'	0.51	0.56	0.58	0.12	_	_	_		38
					30'	0.61	0.68	0.70	0.12	_	_	-		38
					0.33' - <2'	0.26	0.44	0.42	0.24	0.24	0.24	0.26		-
				4	2' - <3'	0.26	0.44	0.42	0.12	-	-	-		43
				7	3' - <5'	0.22	0.30	0.33	0.12	_	_	_		43
6' x 5'	10	10	10	to	5' - 10'	0.24	0.25	0.27	0.12	_	_	_		39
2 / 3	'		1	'0	15'	0.33	0.36	0.39	0.12	_	_	_		38
				12	20'	0.42	0.48	0.53	0.12	_	_	_		38
				12	25'	0.42	0.48	0.63	0.12	_	_	_		38
					30'	0.61	0.74	0.76	0.12	_	_	_		38
					0.33' - <2'	0.01	0.46	0.70	0.12	0.24	0.24	0.27		-
				_	2' - <3'	0.27	0.46	0.44	0.24	-				52
				4	2 - <3 3' - <5'	0.27	0.40	0.44	0.12	_	_	-		52
6' v 6'	10	10	10		5' - 10'	0.25	0.31	0.34						43
6' x 6'	10	10	10	to					0.12	-	_	-		39
				,	15'	0.35	0.39	0.42	0.12	-		-		
				12	20'	0.45	0.52	0.55	0.12	-	-	-		39
					25'	0.54	0.65	0.68	0.12	-	_	_		38
			<u> </u>		30'	0.64	0.78	0.81	0.12	-	-	_		38

≥ DESCRIPTION: LAST REVISION 07/01/13

TABL	E 11A	- ST	ANDA	RD PRI	CAST BOX	CULV	ERT I	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPANS	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	BOT.	_	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(5)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.42	0.58	0.52	0.22	0.22	0.31	0.42		-
				4	2' - <3'	0.42	0.58	0.51	0.11	-	-	-		43
					3' - <5'	0.36	0.41	0.44	0.11	-	-	-		43
7' x 4'	9	9	9	to	5' - 10'	0.39	0.40	0.39	0.11	-	-	-		43
					15'	0.56	0.56	0.58	0.11	-	-	-		41
				12	20'	0.74	0.76	0.77	0.11	-	-	-		41
					25'	0.92	0.97	0.97	0.11	-	-	_		41
	9	9.5	9	7 to 12	30'	1.09	1.18	1.10	0.11	-	-	-		41
					0.33' - <2'	0.41	0.61	0.55	0.22	0.23	0.30	0.41		-
				4	2' - <3'	0.41	0.61	0.55	0.11	-	-	-		47
					3' - <5'	0.37	0.43	0.47	0.11	-	-	-		43
7' x 5'	9	9	9	to	5' - 10'	0.39	0.41	0.43	0.11	-	-	-		43
					15'	0.56	0.61	0.63	0.11	-	-	-	-5	41
				12	20'	0.73	0.82	0.83	0.11	-	-	-	te	41
					25'	0.90	1.04	1.06	0.11	-	-	-	No	41
	9	9.5	9	7 to 12	30'	1.06	1.26	1.19	0.11	-	-	-	General Note	41
					0.33' - <2'	0.42	0.63	0.58	0.22	0.24	0.30	0.42	ner	-
				4	2' - <3'	0.42	0.63	0.58	0.11	-	-	-	Ge.	59
					3' - <5'	0.38	0.45	0.50	0.11	-	-	-	See	47
7' x 6'	9	9	9	to	5' - 10'	0.41	0.44	0.47	0.11	-	-	-	Š	43
					15'	0.57	0.65	0.68	0.11	-	-	-		41
				12	20'	0.75	0.87	0.90	0.11	-	-	-		41
					25'	0.93	1.11	1.13	0.11	-	-	-		41
	9	9.5	9	7 to 12	30'	1.07	1.35	1.27	0.11	-	-	-		41
					0.33' - <2'	0.44	0.66	0.61	0.22	0.25	0.31	0.44		-
				4	2' - <3'	0.44	0.65	0.61	0.11	-	-	-		59
					3' - <5'	0.41	0.47	0.52	0.11	-	-	-		59
7' x 7'	9	9	9	to	5' - 10'	0.44	0.47	0.52	0.11	-	-	-		47
					15'	0.62	0.69	0.74	0.11	-	-	_		43
				12	20'	0.80	0.93	0.97	0.11	-	-	-		43
					25'	0.99	1.18	1.22	0.11	-	-	-		43
	9	9.5	9	7 to 12	30'	1.12	1.43	1.36	0.11	-	-	-		41

TABI	LE 11B	B - ST	ANDA	RD PRI	ECAST BOX	CULV	ERT	DESIG	GNS (.	3" CO	VER)	- 7' 5	SPANS	5
SPAN x RISE	SLAB	7 WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15		As1 EXT.
(S) (R)	TOP	вот.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0V E									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.33	0.49	0.44	0.24	0.24	0.24	0.33		-
				4	2' - <3'	0.33	0.49	0.44	0.12	-	-	-		43
					3' - <5'	0.29	0.35	0.38	0.12	-	-	-		43
7' x 4'	10	10	10	to	5' - 10'	0.31	0.30	0.31	0.12	-	-	-		43
					15'	0.44	0.44	0.45	0.12	-	-	-		41
				12	20'	0.58	0.59	0.60	0.12	-	_	-		41
					25'	0.71	0.74	0.75	0.12	-	-	-		41
					30'	0.85	0.91	0.91	0.12	-	-	-		41
					0.33' - <2'	0.32	0.51	0.47	0.24	0.24	0.24	0.32		-
				4	2' - <3'	0.32	0.51	0.47	0.12	-	-	-		47
					3' - <5'	0.29	0.37	0.41	0.12	-	-	-		43
7' x 5'	10	10	10	to	5' - 10'	0.31	0.32	0.35	0.12	-	-	-		43
					15'	0.44	0.47	0.50	0.12	-	-	-	5	41
				12	20'	0.57	0.63	0.65	0.12	-	-	-	ıte	41
					25'	0.70	0.80	0.82	0.12	-	-	-	Note	41
					30'	0.84	0.97	0.99	0.12	-	-	-	General	41
					0.33' - <2'	0.33	0.53	0.50	0.24	0.24	0.24	0.33	neı	-
				4	2' - <3'	0.33	0.53	0.50	0.12	-	-	-	Ge.	59
					3' - <5'	0.30	0.38	0.43	0.12	-	-	-	өө	47
7' x 6'	10	10	10	to	5' - 10'	0.33	0.35	0.38	0.12	-	-	-	56	43
					15'	0.45	0.51	0.54	0.12	-	-	-		41
				12	20'	0.58	0.68	0.70	0.12	-	-	-		41
					25'	0.72	0.85	0.88	0.12	-	-	-		41
					30'	0.85	1.04	1.06	0.12	-	-	-		41
					0.33' - <2'	0.35	0.55	0.52	0.24	0.24	0.24	0.35		-
				4	2' - <3'	0.35	0.55	0.52	0.12	-	-	-		59
					3' - <5'	0.32	0.40	0.46	0.12	-	-	-		59
7' x 7'	10	10	10	to	5' - 10'	0.35	0.37	0.41	0.12	-	-	-		47
					15'	0.48	0.54	0.58	0.12	-	-	-		43
				12	20'	0.62	0.72	0.76	0.12	-	_	-		43
					25'	0.76	0.90	0.94	0.12	-	-	-		43
					30'	0.90	1.10	1.13	0.12	-	-	-		41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

LAST REVISION 07/01/13

FDOT

TABL	.E 12/	\ - <i>ST</i>	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIG	GNS (3" CC	VER)	- 8' .	SPAN	S
SPAN x RISE		/ WAL	L THIC	KNESS	DESIGN			R	EINFOR			15		As1 EXT.
(S) (R)	TOP	BOT.	l	HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
(5+)	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.52	0.66	0.57	0.22	0.24	0.42	0.52		_
				7	2' - <3'	0.52	0.66	0.57	0.11	-	-	-		50
				to	3' - <5'	0.48	0.49	0.52	0.11	-	-	-		50
8' x 4'	9	9	9	10	5' - 10'	0.52	0.48	0.49	0.11	-	-	-		45
				12	15'	0.75	0.72	0.72	0.11	-	-	-		41
				12	20'	1.00	0.98	0.97	0.11	-	-	-		41
	9	9.5	9	8 to	25'	1.25	1.24	1.14	0.11	-	-	-		41
	10	10.5	9	12	30'	1.31	1.29	1.21	0.11	-	-	-		41
				4	0.33' - <2'	0.51	0.69	0.60	0.22	0.25	0.40	0.51		-
				7	2' - <3'	0.51	0.69	0.60	0.11	-	-	-		50
				to	3' - <5'	0.46	0.52	0.56	0.11	-	-	-		45
8' x 5'	9	9	9	10	5' - 10'	0.51	0.51	0.53	0.11	-	-	_		45
				12	15'	0.74	0.77	0.78	0.11	-	-	_		41
				12	20'	0.97	1.05	1.05	0.11	-	-	-		41
	9	9.5	9	8 to	25'	1.20	1.33	1.23	0.11	-	-	-		41
	10	10.5	9	12	30'	1.26	1.38	1.30	0.11	-	-	-		41
				4	0.33' - <2'	0.51	0.72	0.64	0.22	0.26	0.39	0.51	5	_
				4	2' - <3'	0.51	0.72	0.64	0.11	-	-	-	te	50
					3' - <5'	0.47	0.55	0.59	0.11	-	-	-	Note	50
8' x 6'	9	9	9	to	5' - 10'	0.52	0.55	0.58	0.11	-	-	-		45
				4.0	15'	0.74	0.83	0.85	0.11	-	-	-	ıer	41
				12	20'	0.97	1.12	1.13	0.11	-	-	-	General	41
	9	9.5	9	8 to	25'	1.18	1.42	1.32	0.11	-	-	-) (41
	10	10.5	9	12	30'	1.26	1.46	1.39	0.11	-	-	_	See	41
				_	0.33' - <2'	0.52	0.74	0.67	0.22	0.26	0.40	0.52		_
				4	2' - <3'	0.52	0.74	0.67	0.11	_	-	-		55
					3' - <5'	0.49	0.57	0.62	0.11	_	_	_		55
8' x 7'	9	9	9	to	5' - 10'	0.55	0.59	0.63	0.11	_	_	_		50
					15'	0.77	0.88	0.91	0.11	_	_	_		41
				12	20'	1.01	1.19	1.21	0.11	_	-	-		41
	9	9.5	9	8 to	25'	1.21	1.51	1.41	0.11	_	_	_		41
	10	10.5	9	12	30'	1.31	1.53	1.47	0.11	_	_	_		41
		10.5			0.33' - <2'	0.55	0.77	0.70	0.22	0.27	0.41	0.55		_
				4	2' - <3'	0.55	0.77	0.70	0.13	-	-	-		65
					3' - <5'	0.53	0.59	0.64	0.12	_	_	_		65
8' x 8'	9	9	9	to	5' - 10'	0.60	0.63	0.68	0.11	_	_	_		55
	2				15'	0.83	0.93	0.98	0.11	_	_	_		45
				12	20'	1.08	1.26	1.29	0.11	_	_	_		45
	9	9.5	9	8 to	25'	1.28	1.59	1.50	0.11	_	_	_		41
	10	10.5	9	12	30'	1.41	1.61	1.55	0.11	_	_	_		41
	10	10.5	<u> </u>	14		1.71	1.01	1.55	0.11		<u> </u>			71

					ECAST BOX	CULV	ERT						SPAN.	
SPAN x RISE (S) (R)	TOP	ВОТ.	SIDE		DESIGN EARTH COVER			R		RCEMEN q. in./F	T AREA t.)	15		As1 EX LENGT
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw)	(H) (in.)	ABOVE TOP SLAB									(M) (in.)
(, c.,	(111.)	(111.)	(111.)	(111.)		As 1	As2	As3	As4	As5	As7	A58	As9	(,,,,
					0.33' - <2'	0.42	0.56	0.49	0.24	0.24	0.32	0.41		-
				4	2' - <3'	0.42	0.56	0.49	0.12	-	-	-		50
G/ #/					3' - <5'	0.38	0.42	0.46	0.12	-	-	-		50
8' x 4'	10	10	10	to	5' - 10'	0.41	0.38	0.39	0.12	-	-	-		45
					15'	0.59	0.56	0.57	0.12	-	-	-		41
				12	20'	0.78	0.75	0.76	0.12	-	-	-		41
	1.0	10.5	1.0	0 / 10	25'	0.97	0.96	0.96	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.15	1.16	1.10	0.12	-	-	-		41
					0.33' - <2'	0.40	0.58	0.52	0.24	.034	0.31	0.40		-
				4	2' - <3'	0.40	0.58	0.52	0.12	-	-	-	-	50
0/ 5/	1.0	1.0	1.0		3' - <5'	0.37	0.45	0.48	0.12	-	-	-		45
8' x 5'	10	10	10	to	5' - 10'	0.41	0.41	0.43	0.12	-	-	-		45
					15'	0.58	0.60	0.62	0.12	-	-	-		41
				12	20'	0.76	0.81	0.81	0.12	-	-	-		41
	10	10.5	1.0	0 . 10	25'	0.94	1.03	1.03	0.12	-	-	-		41
	10	10.5	10	8 to 12	30'	1.10	1.24	1.24	0.12	-	-	-		41
					0.33' - <2'	0.40	0.60	0.55	0.24	0.24	0.30	0.40	5 5	-
				4	2' - <3'	0.40	0.60	0.55	0.12	-	-	-	Note	50
01 61	1.0	1.0	1.0		3' - <5'	0.37	0.47	0.51	0.12	-	-	-		50
8' x 6'	10	10	10	to	5' - 10'	0.42	0.43	0.46	0.12	-	-	-	General	45
					15'	0.58	0.64	0.67	0.12	-	-	-	ene	41
				12	20'	0.76	0.86	0.88	0.12	-	-	-		41
	10	10.5	1.0	0 / 12	25' 30'	0.94	1.09	1.11	0.12	-	-	-	See	41
	10	10.5	10	8 to 12		1.09	1.32	1.26	0.12	-	- 0.20	- 0.41	, ,	41
					0.33' - <2'	0.41	0.63	0.58	0.24	0.24	0.30	0.41		-
				4	2' - <3' 3' - <5'	0.41	0.63	0.58	0.12	-	_	-		55 55
8' x 7'	10	10	10	l .	5' - 10'		0.49	0.53	0.12			_		50
0 X /	10	10	10	to	15'	0.44	0.48	0.30	0.12	-	_	_	-	45
				1.2	20'	0.61	0.08	0.72	0.12	_	_	-		43
				12	25'	0.78	1.16	1.18	0.12	_	_	_	-	41
	10	10.5	10	8 to 12	30'	1.11	1.40	1.34	0.12	_	_	_		41
	10	10.5	10	0 10 12	0.33' - <2'	0.44	0.64	0.60	0.12	0.24	0.31	0.44	-	- 41
				,	2' - <3'	0.44	0.64	0.60	0.24	0.24	0.51	0.44	-	65
				4	3' - <5'	0.44	0.51	0.56	0.12	_	_	_		65
8' x 8'	10	10	10		5' - 10'	0.42	0.51	0.55	0.12	_	_	_	-	55
0 1 0	10	10	10	to	3 - 10 15'	0.47	0.50	0.55	0.12	_	_	_	-	45
				1.2	20'	0.84	0.72	1.01	0.12	_	_	_	-	45
				12	25'	1.03	1.22	1.01	0.12	-	-	-	-	43
	10	10.5	10	8 to 12	30'	1.16	1.47	1.42	0.12	_	_	_	-	41
	10	10.5	10	10 10 12	J 30	1.10	1.4/	1.4∠	0.12	_	_	_	I	41

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: REVISION 07/01/13

TABLI	E 13A	- <i>ST</i>	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (.	3" CC	VER)	- 9' 5	SPAN	S
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	ıS		As1 EXT.
(S) (R)	TOP	BOT.		HAUNCH	EARTH COVER				(5	q. in./F	t.)			LENGTH
4	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
					0.33' - <2'	0.62	0.78	0.65	0.22	0.26	0.52	0.61		-
				4	2' - <3'	0.62	0.78	0.65	0.11	-	-	-		54
				to	3' - <5'	0.58	0.63	0.61	0.11	-	-	-		49
9' x 5'	9	9	9	12	5' - 10'	0.65	0.63	0.64	0.11	-	-	-		49
					15'	0.95	0.96	0.95	0.11	-	-	-		44
	9	9	9	8	20'	1.26	1.32	1.28	0.11	-	-	-		44
	10	10.5	9	to	25'	1.39	1.41	1.32	0.11	-	-	-		44
	11	11.5	9	12	30'	1.46	1.50	1.42	0.11	-	-	-		44
					0.33' - <2'	0.60	0.81	0.69	0.22	0.27	0.51	0.60		-
				4	2' - <3'	0.60	0.81	0.69	0.11	-	-	-		54
				to	3' - <5'	0.56	0.66	0.65	0.11	-	_	_		49
9' x 6'	9	9	9	12	5' - 10'	0.65	0.68	0.69	0.11	-	-	-		49
					15'	0.94	1.03	1.02	0.11	-	-	-		44
	9	9	9	8	20'	1.25	1.40	1.38	0.11	-	-	-		44
l L	10	10.5	9	to	25'	1.37	1.49	1.40	0.11	1	1	1		44
	11	11.5	9	12	30'	1.44	1.58	1.50	0.11	ı	ı	1		44
					0.33' - <2'	0.61	0.84	0.72	0.22	0.28	0.51	0.61	5	-
				4	2' - <3'	0.61	0.83	0.72	0.11	1	1	ı	Note	59
				to	3' - <5'	0.58	0.69	0.68	0.11	-	-	1		54
9' x 7'	9	9	9	12	5' - 10'	0.67	0.73	0.75	0.11	1	ı	1	General	49
					15'	0.96	1.09	1.10	0.11	1	-	1	neı	44
	9	9	9	8	20'	1.27	1.49	1.47	0.11	1	-	1	<i>Ge</i>	44
	10	10.5	9	to	25'	1.38	1.57	1.48	0.11	-	-	-	See	44
	11	11.5	9	12	30'	1.49	1.70	1.58	0.11	-	_	-	Sé	44
	9	9.5	9		0.33' - <2'	0.60	0.85	0.73	0.22	0.29	0.52	0.53		-
				4	2' - <3'	0.64	0.86	0.76	0.12	-	-	1		59
				to	3' - <5'	0.62	0.72	0.72	0.11	-	-	1		59
9' x 8'	9	9	9	12	5' - 10'	0.71	0.77	0.81	0.11	-	-	-		54
					15'	1.01	1.16	1.17	0.11	-	_	1		44
	9	9.5	9	8	20'	1.27	1.56	1.45	0.11	ı	ı	1		44
	10	10.5	9	to	25'	1.45	1.65	1.57	0.11	-	-	1		44
	11	11.5	9	12	30'	1.59	1.72	1.66	0.11	-	-	1		44
	9	9.5	9		0.33' - <2'	0.68	0.88	0.76	0.22	0.29	0.55	0.57		-
				4	2' - <3'	0.68	0.88	0.78	0.18	-	_	-		72
				to	3' - <5'	0.68	0.75	0.78	0.18	-	_	-		72
9' x 9'	9	9	9	12	5' - 10'	0.79	0.82	0.88	0.17	-	-	1		59
					15'	1.11	1.22	1.26	0.13	-	-	1		49
	9	9.5	9	8	20'	1.37	1.64	1.54	0.13	-	-	-		49
	10	10.5	9	to	25'	1.56	1.73	1.65	0.13	-	-	-		44
. ⊢	11	11.5	9.5	12	30'	1.56	1.73	1.68	0.12					44

TABL	LE 13E	3 - ST	ANDA	ARD PR	ECAST BOX	CULV	'ERT	DESIC	GNS (3" CC	VER)	<i>- 9'</i> .	SPAN	S
SPAN x RISE	SLAE	/ WAL			DESIGN	REINFORCEMENT AREAS							As1 EXT	
(S) (R)	TOP	BOT.	1	HAUNCH	EARTH COVER									LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB				1		1			(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As7	As8	As9	(111.)
				4	0.33' - <2'	0.49	0.65	0.57	0.24	0.24	0.40	0.48		-
				4	2' - <3'	0.49	0.65	0.57	0.12	-	-	-		54
	10	10	10	to	3' - <5'	0.46	0.54	0.53	0.12	-	-	-		49
9' x 5'	10	10	10	"	5' - 10'	0.52	0.50	0.51	0.12	-	-	-		49
				12	15'	0.75	0.74	0.75	0.12	-	-	-		44
					20'	0.98	1.01	1.00	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.21	1.27	1.19	0.12	-	-	-		44
	11	11.5	10	12	30'	1.30	1.36	1.30	0.12	-	-	-		44
				4	0.33' - <2'	0.48	0.68	0.60	0.24	0.24	0.39	0.48		-
				4	2' - <3'	0.48	0.68	0.60	0.12	-	-	-		54
	10	10	10	+ -	3' - <5'	0.45	0.57	0.56	0.12	-	-	-		49
9' x 6'	10	10	10	to	5' - 10'	0.52	0.53	0.56	0.12	-	-	-		49
				12	15'	0.74	0.79	0.81	0.12	-	-	-		44
				12	20'	0.97	1.07	1.07	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.18	1.35	1.28	0.12	-	-	-		44
	11	11.5	10	12	30'	1.27	1.44	1.38	0.12	-	-	-		44
					0.33' - <2'	0.49	0.70	0.63	0.24	0.24	0.39	0.49	General Note 5	-
				4	2' - <3'	0.49	0.70	0.63	0.12	-	-	-		59
					3' - <5'	0.46	0.59	0.59	0.12	-	-	-	N	54
9' x 7'	10	10	10	to	5' - 10'	0.54	0.57	0.60	0.12	-	-	-	le.	49
					15'	0.75	0.84	0.86	0.12	-	_	-	19L	44
				12	20'	0.98	1.13	1.14	0.12	-	-	-	Ge.	44
	10	10.5	10	8 to	25'	1.18	1.43	1.36	0.12	-	-	-	See	44
	11	11.5	10	12	30'	1.28	1.52	1.46	0.12	-	-	-	Se	44
					0.33' - <2'	0.51	0.72	0.65	0.24	0.24	0.39	0.51		-
				4	2' - <3'	0.51	0.72	0.65	0.12	-	-	-		59
					3' - <5'	0.49	0.61	0.62	0.12	-	_	-		59
9' x 8'	10	10	10	to	5' - 10'	0.57	0.60	0.65	0.12	-	-	-		54
					15'	0.79	0.89	0.92	0.12	-	_	-		44
				12	20'	1.02	1.20	1.22	0.12	-	-	-		44
	10	10.5	10	8 to	25'	1.21	1.50	1.44	0.12	-	-	-		44
	11	11.5	10	12	30'	1.33	1.59	1.54	0.12	-	-	-		44
					0.33' - <2'	0.54	0.74	0.68	0.24	0.24	0.41	0.54		_
				4	2' - <3'	0.54	0.74	0.68	0.15	-	-	-		72
					3' - <5'	0.53	0.63	0.64	0.13	_	_	_		72
9' x 9'	10	10	10	to	5' - 10'	0.62	0.64	0.70	0.12	_	_	_		59
J / J					15'	0.85	0.94	0.99	0.12	_	_	_		49
				12	20'	1.09	1.26	1.29	0.12	_	_	_		49
	10	10.5	10	8 to	25'	1.28	1.56	1.52	0.12	_	_	_		44
	11	11.5	10	12	30'	1.42	1.66	1.66	0.12	_	_	_		44
	1 1 1	ע.וו	10	1 2	1 50	1.42	1.00	1.00	0.12					1 44

- 1. See Sheet 2 for General Notes.
- See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

≥ DESCRIPTION: REVISION 07/01/13

TAB	LE 15	- ST	ANDA	RD PRE	CAST BOX	CULVERT DESIGNS (3" COVER) - 11' SPANS								
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN	REINFORCEMENT AREAS							As1 EXT.	
(S) (R)	TOP	BOT.	I	HAUNCH	EARTH COVER		(sq. in./Ft.)							LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB									(M)
(FL.)	(in.)	(in.)	(in.)	(in.)		As 1	As2	As3	As4	As5	As7	As8	A59	(in.)
				4	0.33' - <2'	0.60	0.66	0.54	0.27	0.27	0.52	0.56		-
					2' - <3'	0.60	0.66	0.54	0.14	-	-	-		62
				to	3' - <5'	0.60	0.61	0.53	0.14	-	-	-		62
11' × 4'	11	11	11		5' - 10'	0.79	0.63	0.62	0.14	-	-	-		55
				12	15'	1.01	0.82	0.79	0.14	-	-	-		55
				_	20'	1.34	1.11	1.06	0.14	-	-	-		55
	12	12	11	8 to	25'	1.52	1.27	1.23	0.14	-	-	-		55
	13.5	13.5	11	12	30'	1.54	1.37	1.34	0.14	-	-	-		50
					0.33' - <2'	0.57	0.71	0.60	0.27	0.27	0.47	0.53		
				4	2' - <3'	0.56	0.71	0.60	0.14	-	-	-		62
	,			to	3' - <5'	0.56	0.67	0.59	0.14	-	-	-		55
11' x 6'	11	11	11	12	5' - 10'	0.73	0.71	0.72	0.14	-	-	-	ļ	55
				8	15'	0.92	0.92	0.91	0.14	-	-	-	ļ	50
	11	11	11		20'	1.21	1.25	1.21	0.14	-	-	-		50
	12	12	11	to	25'	1.37	1.43	1.39	0.14	-	_	-		50
	13.5	13.5	11	12	30'	1.39	1.53	1.50	0.14	- 27	- 0.46	-		50
				,	0.33' - <2'	0.55	0.76	0.66	0.27	0.27	0.46	0.55	5 5	-
				4	2' - <3'	0.55	0.76	0.66	0.14	-	-	-	ote	62
1.11 01	1 1	11	,,	to	3' - <5'	0.54	0.72	0.65	0.14	-	-	-	<	62
11' x 8'	11	11	11	12	5' - 10'	0.73	0.79	0.82	0.14	-	-	-	General Note	55
	1.1	1 1	1 1	8	15'	0.93	1.03	1.03	0.14	-	-	-	ene	50
	11	11	11	to	20'	1.21	1.39	1.36	0.14	-	-	-		50
	12	12.5	11	12	25' 30'	1.34 1.41	1.56 1.66	1.50 1.65	0.14	-	-	-	See	50 50
	13.5	13.5	11	12	0.33' - <2'					0.27	- 0.40		, ,	-
				4		0.60	0.81	0.71	0.27	0.27	0.48	0.60		
					2' - <3' 3' - <5'	0.60	0.81	0.71	0.15	-	-	-		75 69
11' x 10'	11	11	11	to 12	5' - 10'	0.61	0.77 0.88	0.70	0.14	_	_	-		62
11 x 10	11	1 1	11	12	15'	1.01	1.13	1.15	0.14	_	_	_	-	55
	11	11	11	8	20'	1.30	1.13	1.52	0.14	_	_	_		50
}	12	12.5	11	to	25'	1.42	1.70	1.65	0.14	_	_	_		50
	13.5	14	11	12	30'	1.53	1.77	1.74	0.14	_	_	_		50
	10.0	17	11		0.33' - <2'	0.64	0.83	0.74	0.27	0.27	0.51	0.64		-
				4	2' - <3'	0.64	0.83	0.74	0.21	-	-	-		86
				to	3' - <5'	0.67	0.79	0.75	0.21	_	_	_		75
11' × 11'	11	11	11	12	5' - 10'	0.88	0.93	0.99	0.19	_	_	_		69
	**	11	''	•-	15'	1.09	1.19	1.23	0.16	_	_	_		55
	11	11	11	8	20'	1.40	1.59	1.60	0.15	_	_	_		55
	12	12.5	11	to	25'	1.54	1.77	1.73	0.15	_	_	_		50
	13.5	14	11.5	12	30'	1.57	1.77	1.76	0.14	_	_	_		50
	10.0		111.5	-		1.0.		1	,	l	l	I		

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

REVISION 07/01/13

DESCRIPTION:

FDOT

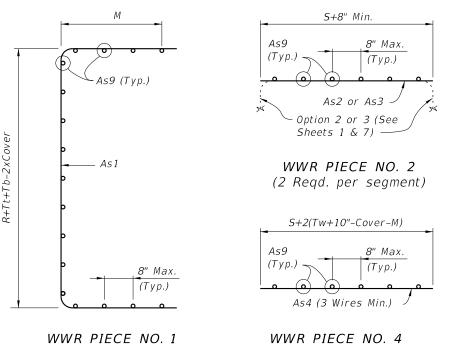
SPAN x RISE	SLAE	3 / WAL	L THIC	KNESS	DESIGN REINFORCEMENT AREAS									As1 EXT
(S) (R)	TOP	вот.	SIDE	HAUNCH	EARTH COVER	R (sq. in./Ft.)							LENGTH	
	(Tt)	(Tb)	(Tw)	(H)	ABOVE									(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As7	As8	As9	(in.)
				4	0.33' - <2'	0.59	0.64	0.51	0.29	0.29	0.52	0.55		_
12' x 4'				to	2' - <3'	0.60	0.64	0.51	0.15	-	-	-		73
	12	12	12		3' - <5'	0.60	0.61	0.51	0.15	-	-	-		66
		12	12		5' - 10'	0.81	0.61	0.61	0.15	-	-	-		66
12 ^ 7				12	15'	1.04	0.80	0.77	0.15	-	-	-		59
				12	20'	1.37	1.08	1.03	0.15	-	-	-		59
	13	13	12	8 to	25'	1.58	1.26	1.21	0.15	-	-	-		59
	14.5	14.5	12	12	30'	1.63	1.38	1.34	0.15	-	-	-		53
				1	0.33' - <2'	0.56	0.70	0.57	029	0.29	0.47	0.52		_
				4 to 12	2' - <3'	0.56	0.70	0.57	0.15	-	-	-		66
	12	12	12		3' - <5'	0.56	0.67	0.57	0.15	-	-	-		59 59
12' x 6'	12	'-			5' - 10'	0.74	0.69	0.70	0.15	-	-	-		
12 % 0					15'	0.94	0.90	0.88	0.15	-	-	-		53
					20'	1.23	1.22	1.17	0.15	-	-	-		53
	13	13	12	8 to	25'	1.40	1.42	1.37	0.15	-	-	-		53
	14.5	15	12	12	30'	1.44	1.54	1.48	0.15	-	-	-		53
	12				0.33' - <2'	0.55	0.75	0.63	0.29	0.29	0.45	0.53 5		_
			12	to	2' - <3'	0.55	0.75	0.63	0.15	-	-	-	Note	66
		12			3' - <5'	0.55	0.73	0.63	0.15	-	-	-	ž	59
12' x 8'	'-	'-	12	12	5' - 10'	0.73	0.77	0.79	0.15	-	-	-	ral	59
12 % 0					15'	0.93	1.00	0.99	0.15	-	-	-	General	53
	12	12	12	8	20'	1.21	1.35	1.31	0.15	-	-	-	Ge	53
	13	13.5	12	to	25'	1.35	1.55	1.48	0.15	-	-	-	See	53
	14.5	15	12	12	30'	1.40	1.67	1.62	0.15	-	-	-	S	53
				4 to	0.33' - <2'	0.57	0.80	0.68	0.29	0.29	0.46	0.57		- 73 66
			12		2' - <3'	0.57	0.80	0.68	0.15	-	-	-		
	12	12			3' - <5'	0.59	0.77	0.68	0.15	-	-	-		
12' x 10'				12	5' - 10'	0.78	0.85	0.89	0.15	-	-	-		59
					15'	0.98	1.10	1.11	0.15	-	-	-		53
	12	12	12	8	20'	1.26	1.47	1.45	0.15	-	-	-		53
	13	13.5	12	to	25'	1.39	1.68	1.63	0.15	-	-	-		53
	14.5	15	12	12	30'	1.48	1.79	1.76	0.15	-	-	-		53
				4 to 12	0.33' - <2'	0.65	0.84	0.73	0.29	0.29	0.50	0.65		_
			12		2' - <3'	0.65	0.84	0.73	0.23	-	-	-		93
	12	12			3' - <5'	0.68	0.81	0.75	0.22	-	-	-		80
12' x 12'					5' - 10'	0.90	0.94	1.01	0.21	-	-	-		73
					15'	1.12	1.20	1.24	0.18	-	-	-		59
	12	12	12	8	20'	1.42	1.60	1.61	0.16	-	-	-		59
	13	13.5	12	to	25'	1.57	1.81	1.78	0.16	-	-	-		53
	14.5	15	12.5	12	30'	1.63	1.86	1.85	0.15	-	-	_		53

TABLE 16 - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 12' SPANS

NOTES:

- 1. See Sheet 2 of 14 for General Notes.
- 2. See Sheet 7 of 14 for Reinforcing Details and dimension locations.

WELDED WIRE REINFORCEMENT BENDING DIAGRAM



WWR PIECE NO. 1 (2 Reqd. per segment) (

WWR PIECE NO. 4 (Tongue Reinforcement) (4 Reqd. per segment) WWR PIECE NO. 3 (2 Reqd. per segment)

Option 2 or 3

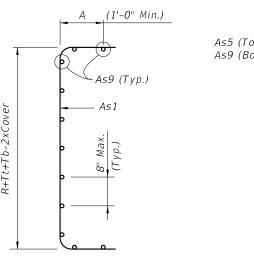
(See Sheets 1

× -

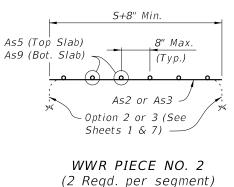
& 7) -

A54

TYPE 2 BOX SECTION (DESIGN EARTH COVER 2' OR GREATER)



WWR PIECE NO. 1 (2 Reqd. per segment)



S+2(Tw+10"-Cover-A)

8" Max.
(Typ.)

As7 (Top Slab)
As8 (Bot. Slab)

WWR PIECE NO. 4 (2 Reqd. per segment) Option 2 or 3
(See Sheets 1
& 7)

As4

As9

As9

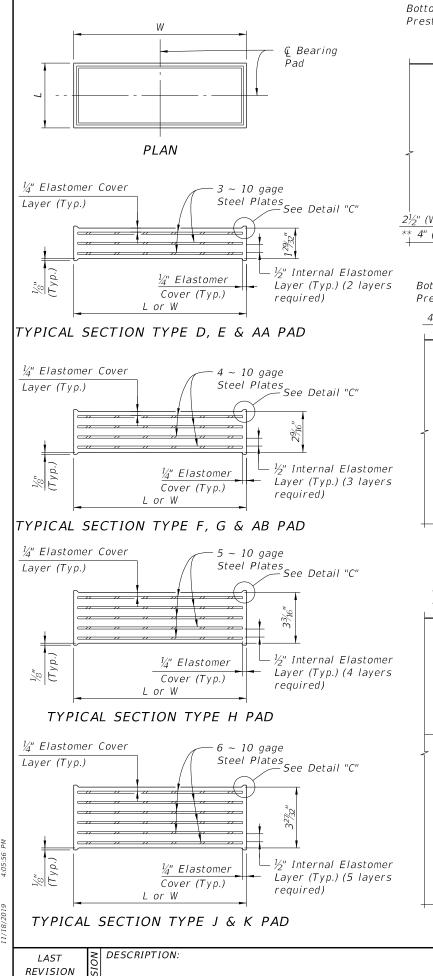
WWR PIECE NO. 3 (2 Reqd. per segment)

TYPE 1 BOX SECTION (DESIGN EARTH COVER LESS THAN 2')

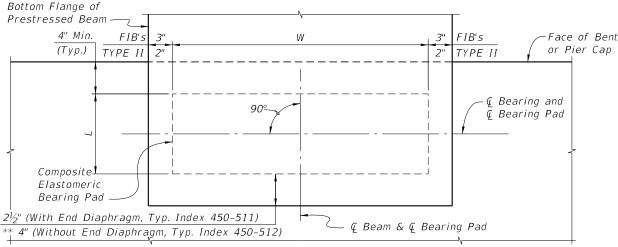
REINFORCEMENT NOTES:

- 1. Reinforcement bending dimensions are out-to-out.
- 2. See General Notes 4, 5 and 6 on Sheet 2.
- 3. See Tables 1 thru 16 for dimensions M, R, S, Tb, Tt and Tw.
- 4. Dimension "A" is determined by the Manufacturer in accordance with the requirements of Detail "B" on Sheets 1 and 7.

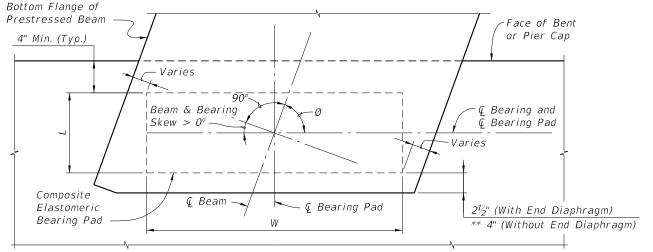
SHEET 14 of 14



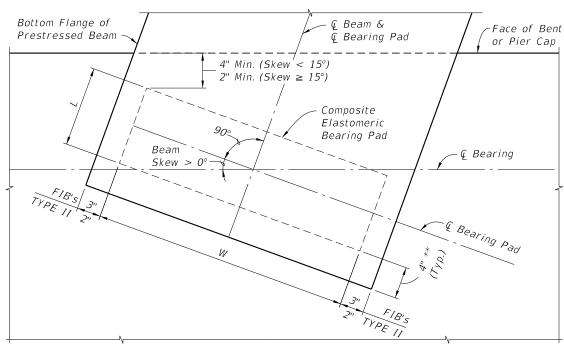
07/01/15



PARTIAL PLAN (Beam & Bearing Skew = 0°)



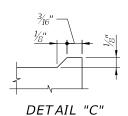
PARTIAL PLAN (Beam & Bearing Skew > 0°) (Use Index 450-511)



PARTIAL PLAN WITH SQUARED END BEAM (Use Index 450-512) (Beam Skew > 0° ; Bearing Skew = 0°)

	BEAM TYPE	BEARIN DIMEN	NG PAD ISIONS	BEARING	ELED G PLATE ISIONS
PAD TYPE (See Note 1)		L	W	С	D
D (G=110psi)		8"	2'-8"	1'-0"	3'-0"
E (G=110psi)		10"	2'-8"	1'-0"	3'-0"
F (G=110psi)	I-BEAM	10"	2'-8"	1'-0"	3'-0"
G (G=150psi)		10"	2'-8"	1'-0"	3'-0"
H (G=150psi)	FLORIDA	10"	2'-8"	1'-0"	3'-0"
J (G=150psi)		10"	2'-8"	1'-0"	3'-0"
K (G=150psi)		1'-0"	2'-8"	1'-1½"	3'-0"
AA (G=110psi)	AASHTO TYPE II	10"	1'-2"	1'-0"	1'-4"
AB (G=150psi)	AAS	10"	1'-2"	1'-0"	1'-4"

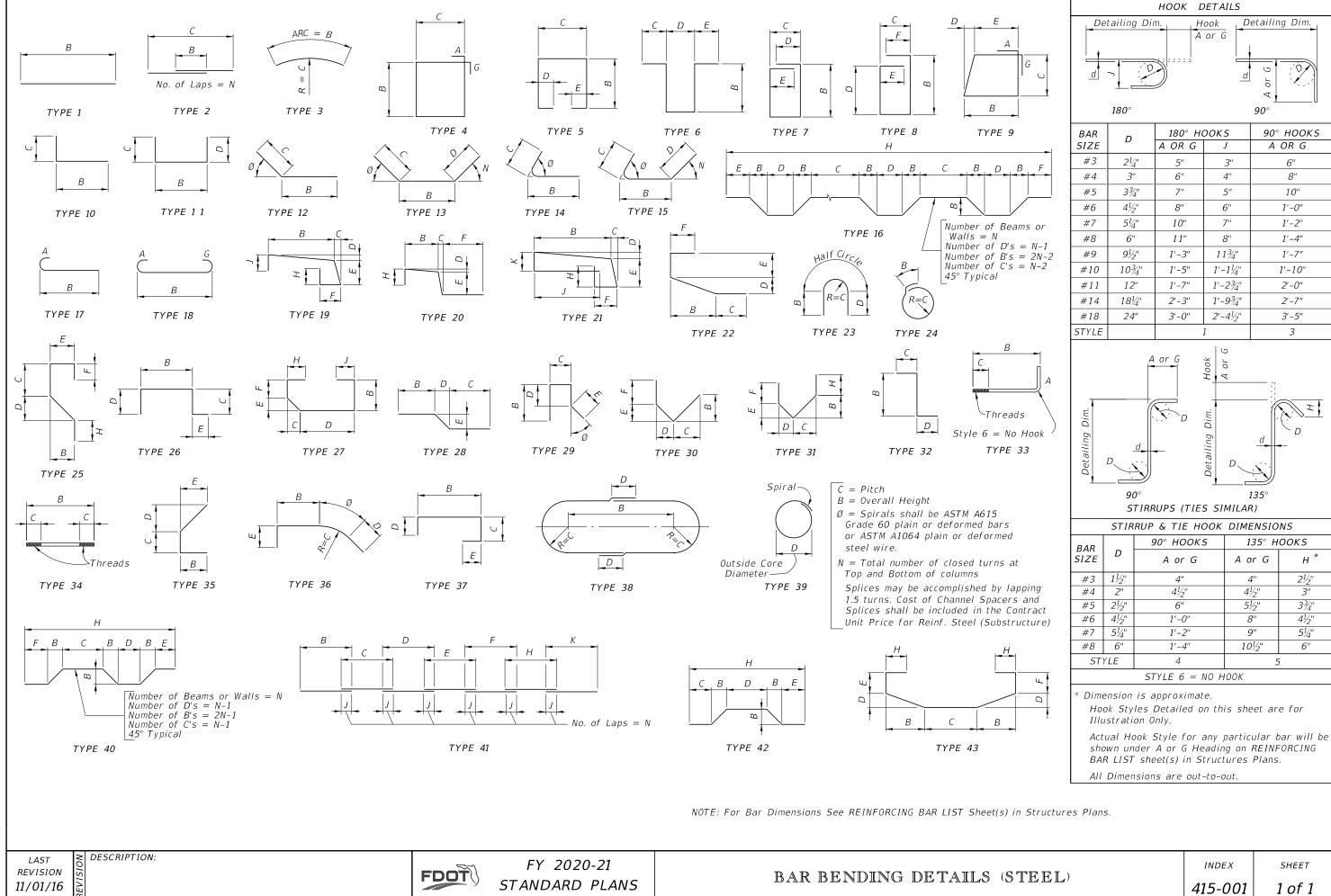
- * Work this sheet with the appropriate type Bearing Plate Detail (See Bearing Plate Data Table) and BEARING PAD DATA TABLE in the Structures Plans. See TABLE OF BEAM VARIABLES and BEARING PLATE DATA TABLE in the Structures Plans for locations where beveled bearing plates are required.
- ** Offset to End of Beam is reduced to 2" for Type K



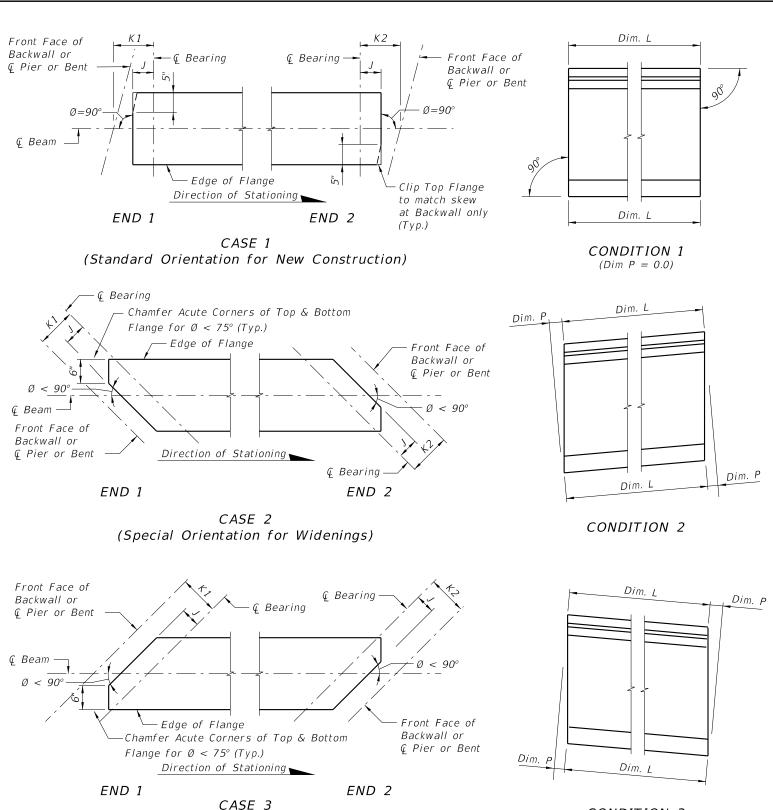
BEARING PAD NOTES:

- 1. Neoprene in Type D, E, F & AA bearing pads shall have a shear modulus (G) of 110 psi. Neoprene in Type G, H, J, K & AB bearing pads shall have a shear modulus (G) of 150 psi.
- 2. Steel Plates in bearing pads shall conform to ASTM A1011 Grade 36, Type 1.
- 3. See Bearing Pad Data Table in Structures Plans for quantities of Type D, E, F, G, H, J, K, AA and/or AB Bearing Pads.

400-510 1 of 1



11/18/2019 4



BEAM NOTES

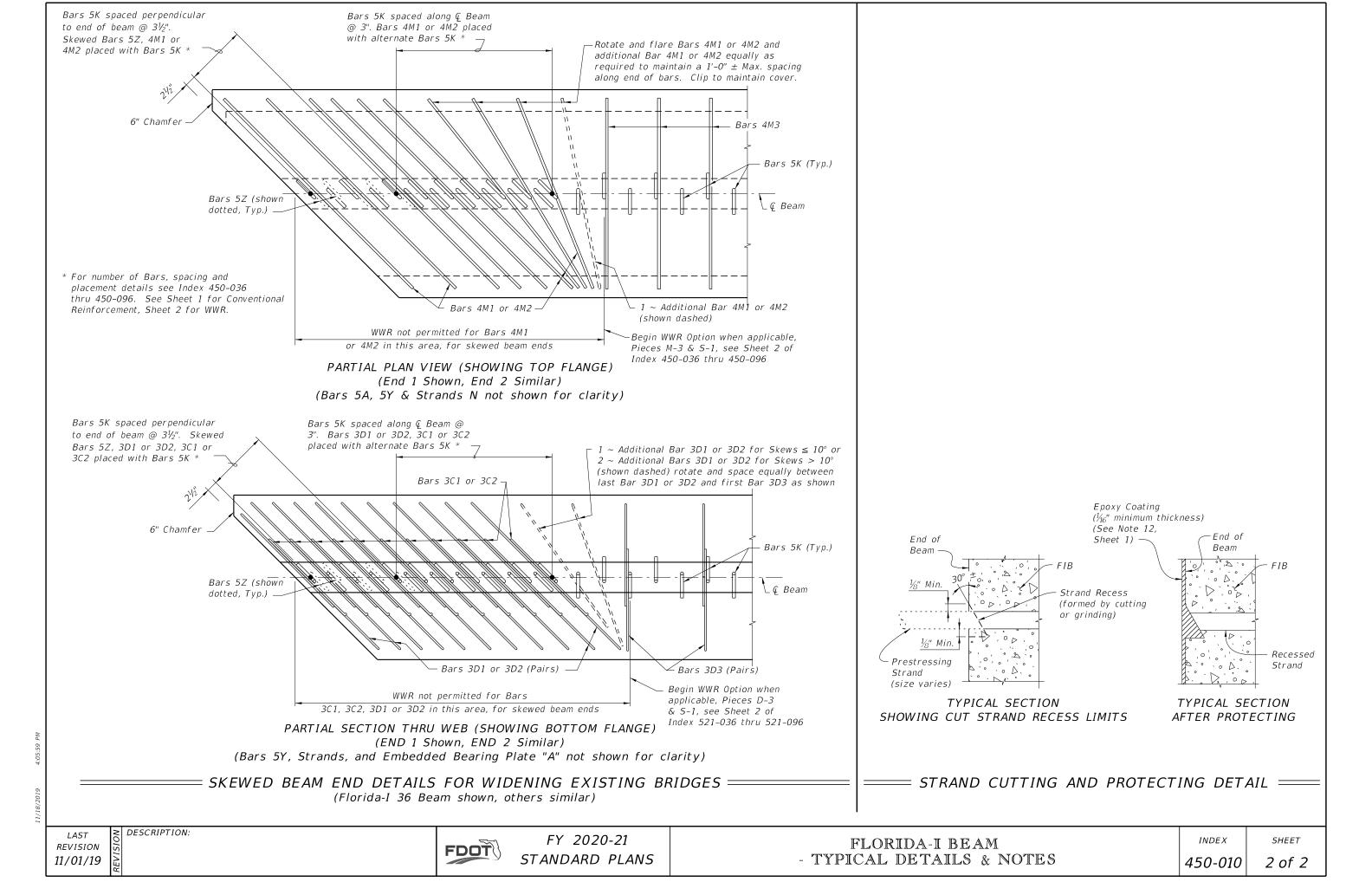
- 1. Work this Index with the Florida-I Beam Standard Details (Index 450-036 thru 450-096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum.
- 4. Strands N: ¾" Ø minimum, stressed to 10,000 lbs. each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
 - A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be extended to facilitate tying to the exterior strands.
 - B. For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
 - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
 - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
 - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.
 - D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement.
- 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
 - B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
 - A. The superstructure environmental classification is slightly or moderately aggressive
 - B. Clear cover to adjacent steel reinforcing is 1"or greater
 - C. Hole inside diameter is 2" maximum
 - D. Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

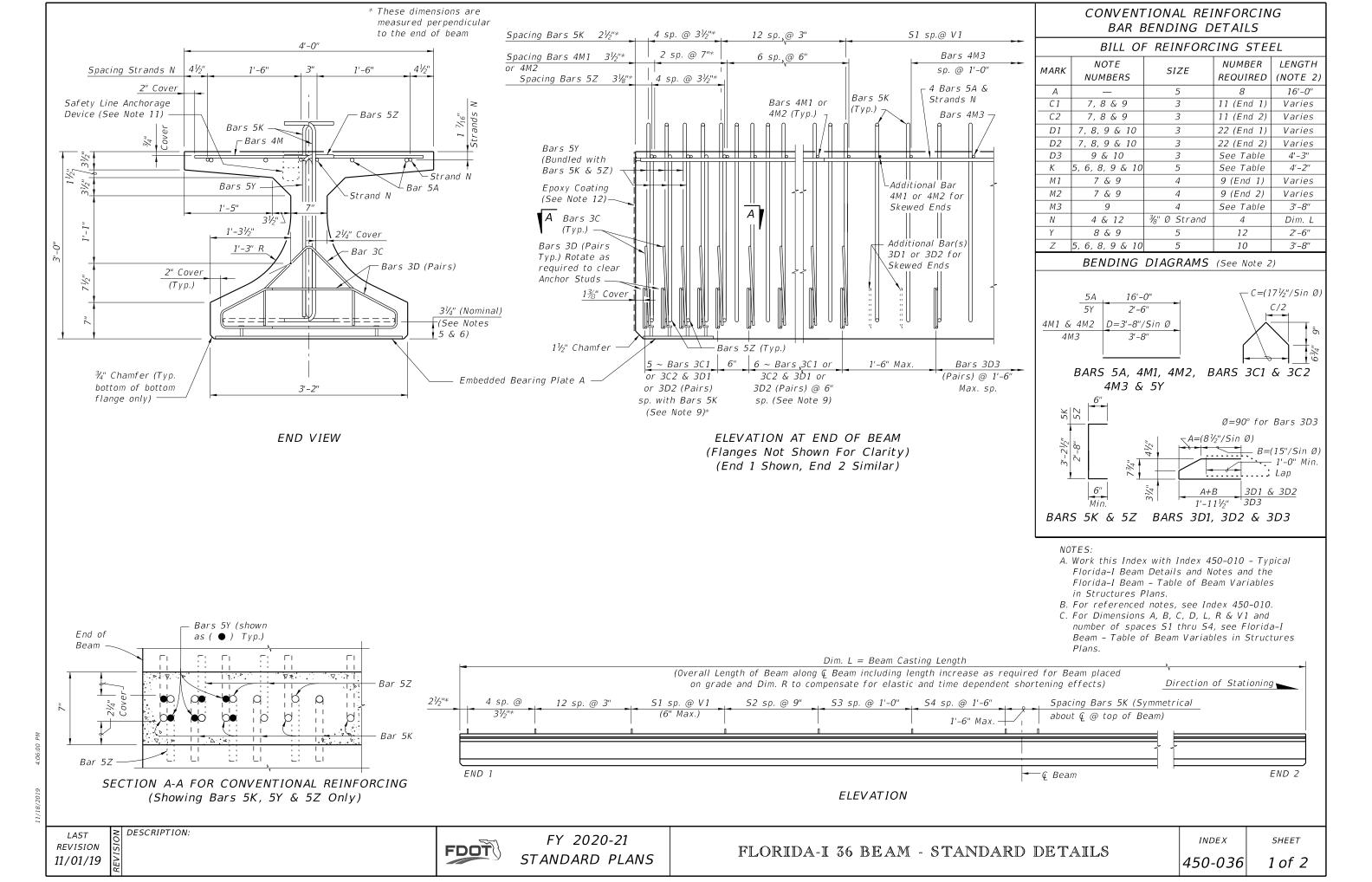
(Special Orientation for Widenings)

SCHEMATIC PLAN VIEWS AT BEAM ENDS

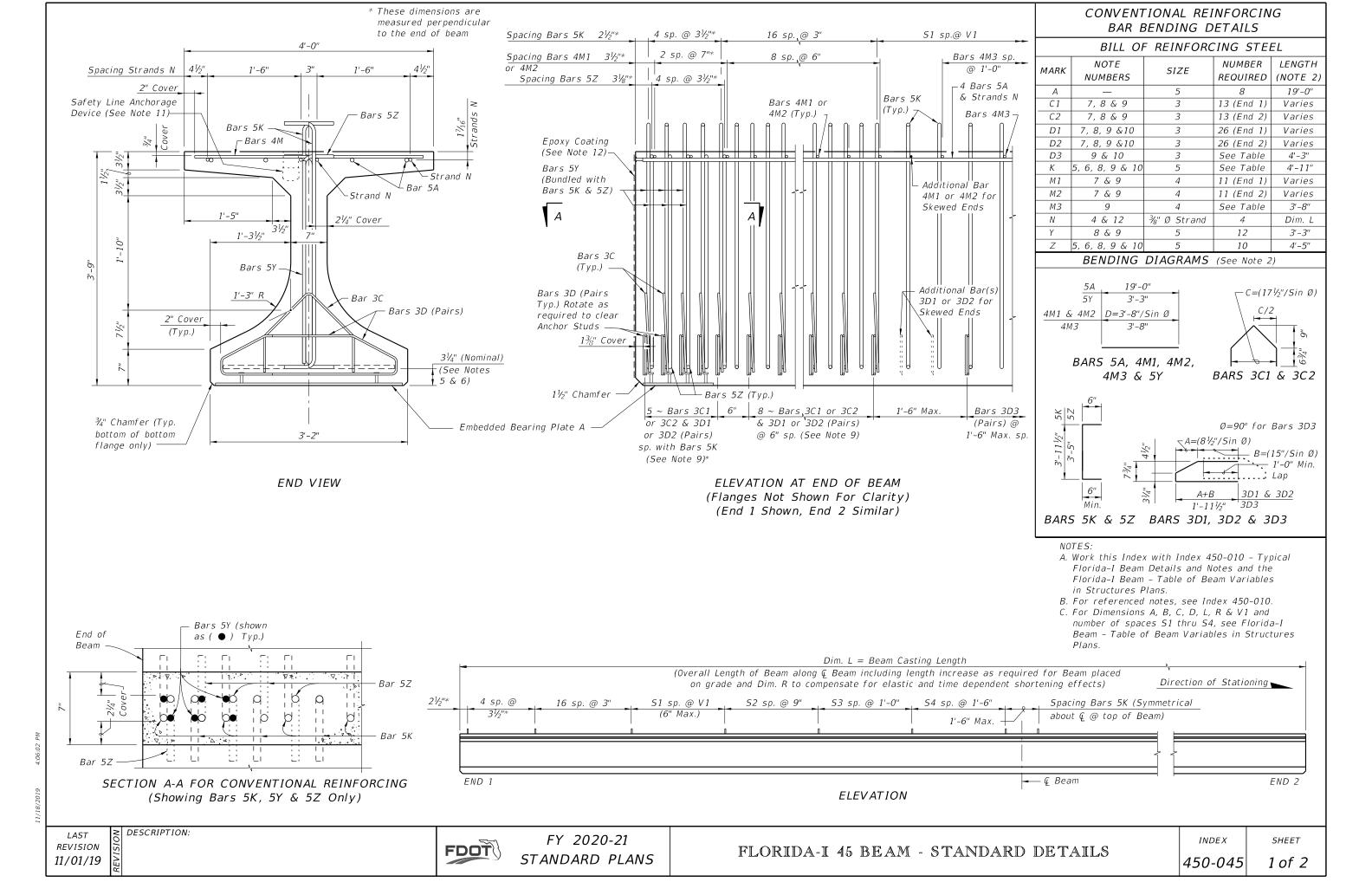
CONDITION 3

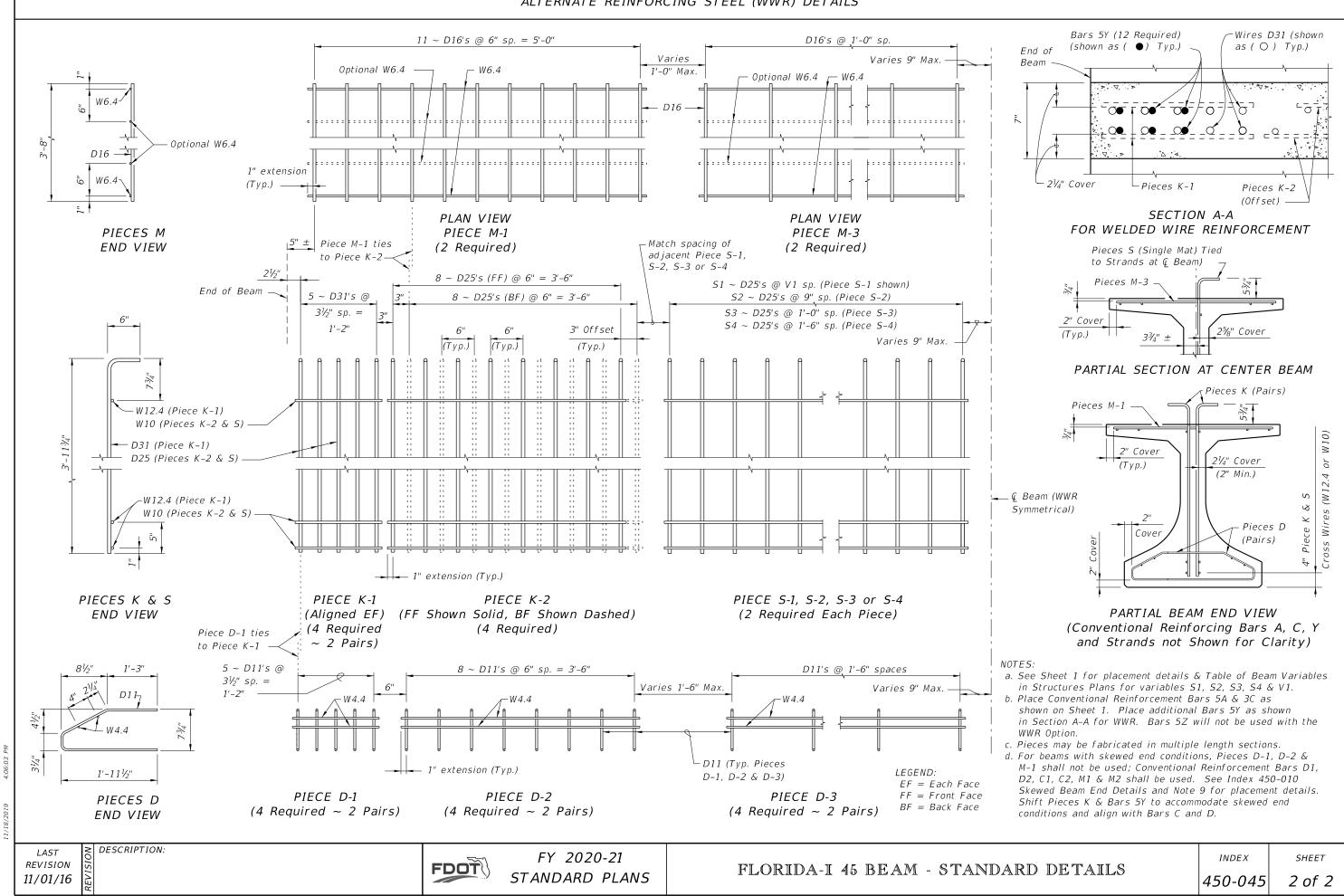
SCHEMATIC END ELEVATIONS OF BEAMS
(Showing Vertical Bevel of Beam End)

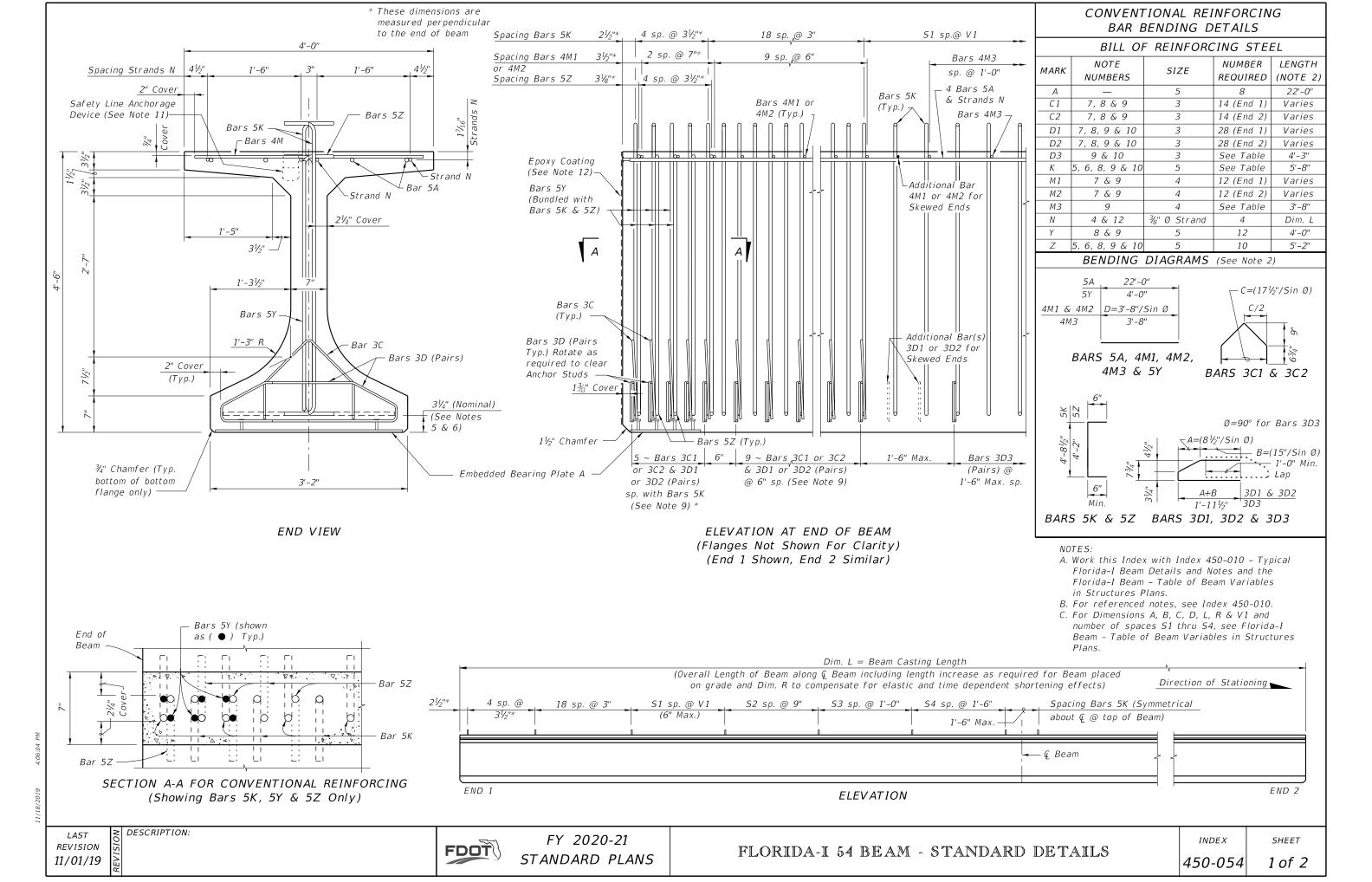


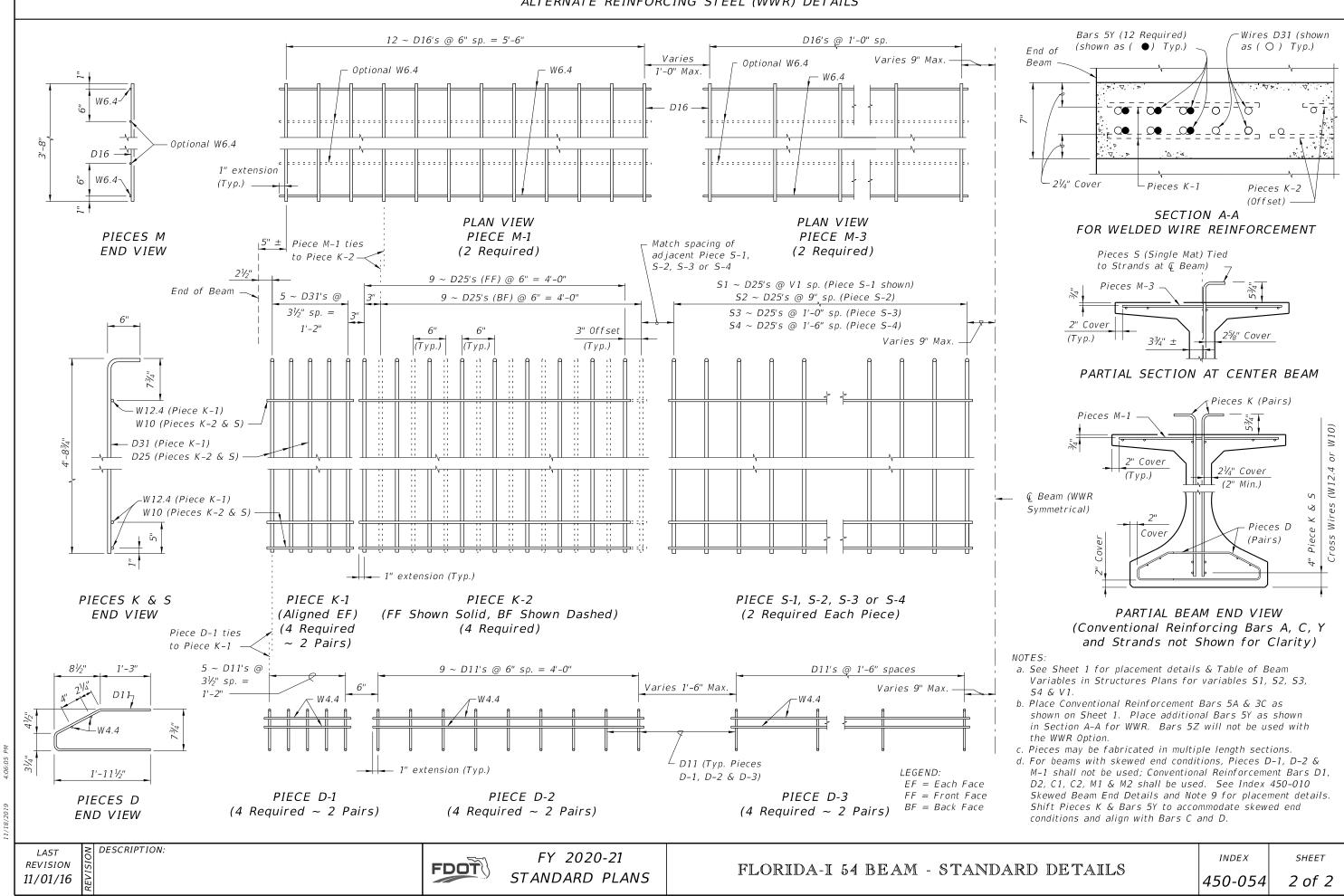


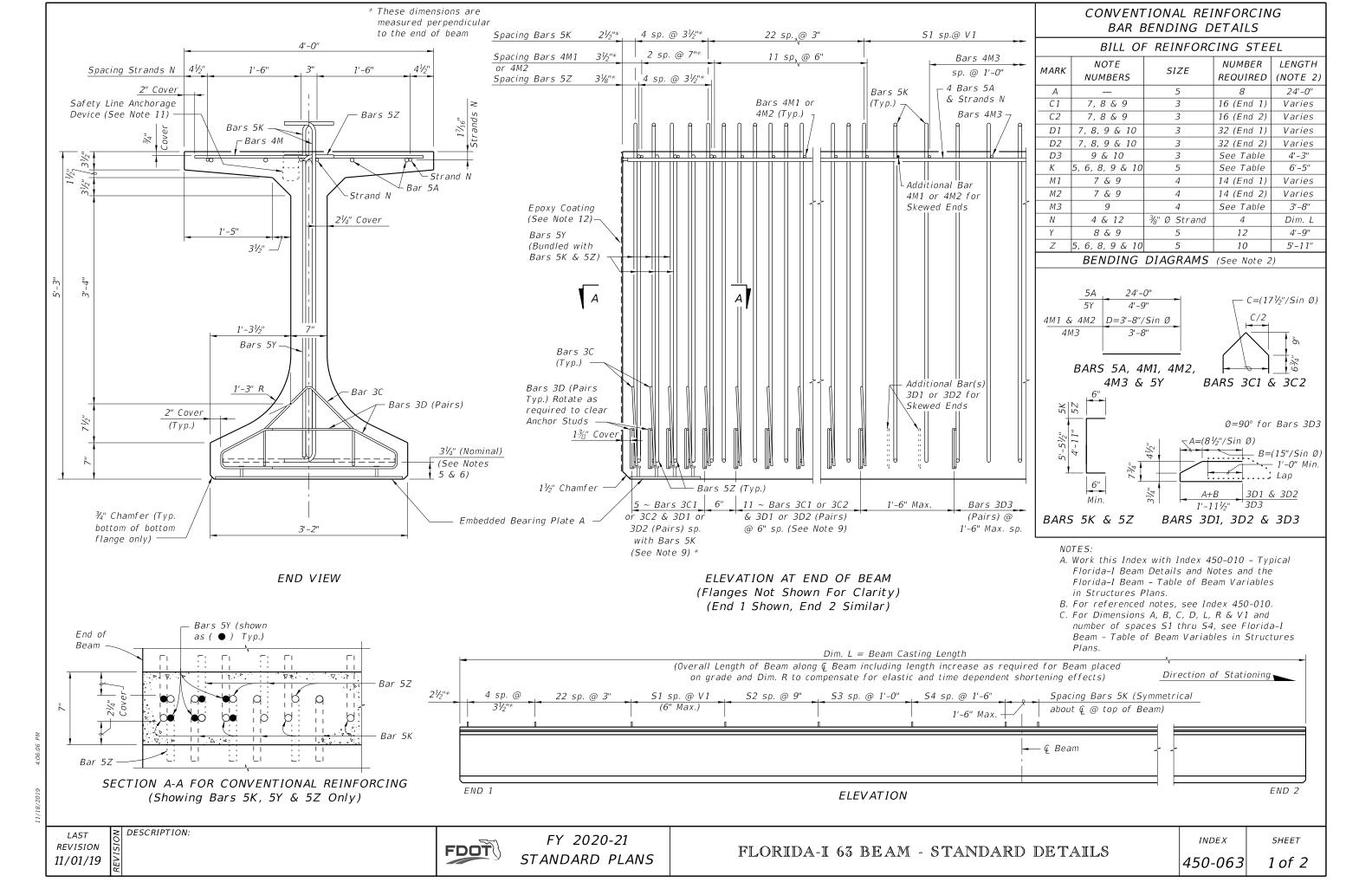
ALTERNATE REINFORCING STEEL (WWR) DETAILS 9 ~ D16's @ 6" sp. = 4'-0" D16's @ 1'-0" sp. Bars 5Y (12 Required) -Wires D31 (shown (shown as (●) Typ.) $as(\bigcirc)$ Typ.) End of Varies Varies 9" Max Optional W6.4 -W6.4 1'-0" Max. Beam — Optional W6.4 - W6.4 W6.4-D16 -Optional W6.4 D16 1" extension W6.4 (Typ.)21/4" Cover Pieces K-1 Pieces K-2 (Offset) PLAN VIEW PLAN VIEW SECTION A-A PIECES M PIECE M-1 PIECE M-3 – Match spacing FOR WELDED WIRE REINFORCEMENT 5" ± Piece M-1 ties END VIEW of adjacent (2 Required) (2 Required) to Piece K-2 Piece S-1, S-2, Pieces S (Single Mat) Tied 6 ~ D25's (FF) S-3 or S-4 21/5" to Strands at © Beam) S1 ~ D25's @ V1 sp. (Piece S-1 shown) End of Beam — Pieces M-3 6 ~ D25's (BF) S2 ~ D25's @ 9" sp. (Piece S-2) @ 6'' = 2'-6'' $3\frac{1}{2}$ " sp. = S3 ~ D25's @ 1'-0" sp. (Piece S-3) S4 ~ D25's @ 1'-6" sp. (Piece S-4) 1'-2" 3" Offset 2" Cover Varies 9" Max. 25/8" Cover (Typ.)(Typ.) (Typ.)(Typ.)3¾" ± PARTIAL SECTION AT CENTER BEAM W12.4 (Piece K-1) Pieces K (Pairs) W10 (Pieces K-2 & S) Pieces M-1 W10) or D31 (Piece K-1 (W12.4 2" Cover D25 (Pieces K-2 & S) -21/4" Cover (Typ.)(2" Min.) Ø Wires (...⊊ Beam (WWR -W12.4 (Piece K-1) Symmetrical) Pieces D W10 (Pieces K-2 & S) Piece Cover (Pairs) 1" extension (Typ.) PARTIAL BEAM END VIEW PIECES K & S PIECE K-1 PIECE K-2 PIECE S-1, S-2, S-3 or S-4 (Conventional Reinforcing Bars A, C, Y END VIEW (Aligned EF) (FF Shown Solid, (2 Required Each Piece) and Strands not Shown for Clarity) (4 Required ~ BF Shown Dashed) Piece D-1 ties to Piece K-1 2 Pairs) (4 Required) NOTES: a. See Sheet 1 for placement details & Table of Beam 5 ~ D11's @ $6 \sim D11's @ 6'' sp. = 2'-6''$ D11's @ 1'-6" spaces 1'-3" Variables in Structures Plans for variables S1, S2, S3, $3\frac{1}{2}$ " sp. = Varies 1'-6" Max. Varies 9" Max 1'-2" b. Place Conventional Reinforcement Bars 5A & 3C as shown on Sheet 1. Place additional Bars 5Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option. c. Pieces may be fabricated in multiple length sections. d. For beams with skewed end conditions, Pieces D-1, D-2 & D11 (Typ. Pieces M-1 shall not be used; Conventional Reinforcement Bars D1, - 1" extension (Typ.) LEGEND: 1'-111/2" D2, C1, C2, M1 & M2 shall be used. See Index 450-010 D-1, D-2 & D-3) EF = Each FaceSkewed Beam End Details and Note 9 for placement details. PIECE D-1 PIECE D-2 PIECE D-3 FF = Front Face Shift Pieces K & Bars 5Y to accommodate skewed end PIECES D BF = Back Faceconditions and align with Bars C and D. (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) (4 Required ~ 2 Pairs) **END VIEW** DESCRIPTION: FY 2020-21 INDEX SHEET REVISION FDOT FLORIDA-I 36 BEAM - STANDARD DETAILS STANDARD PLANS 11/01/16 450-036 2 of 2

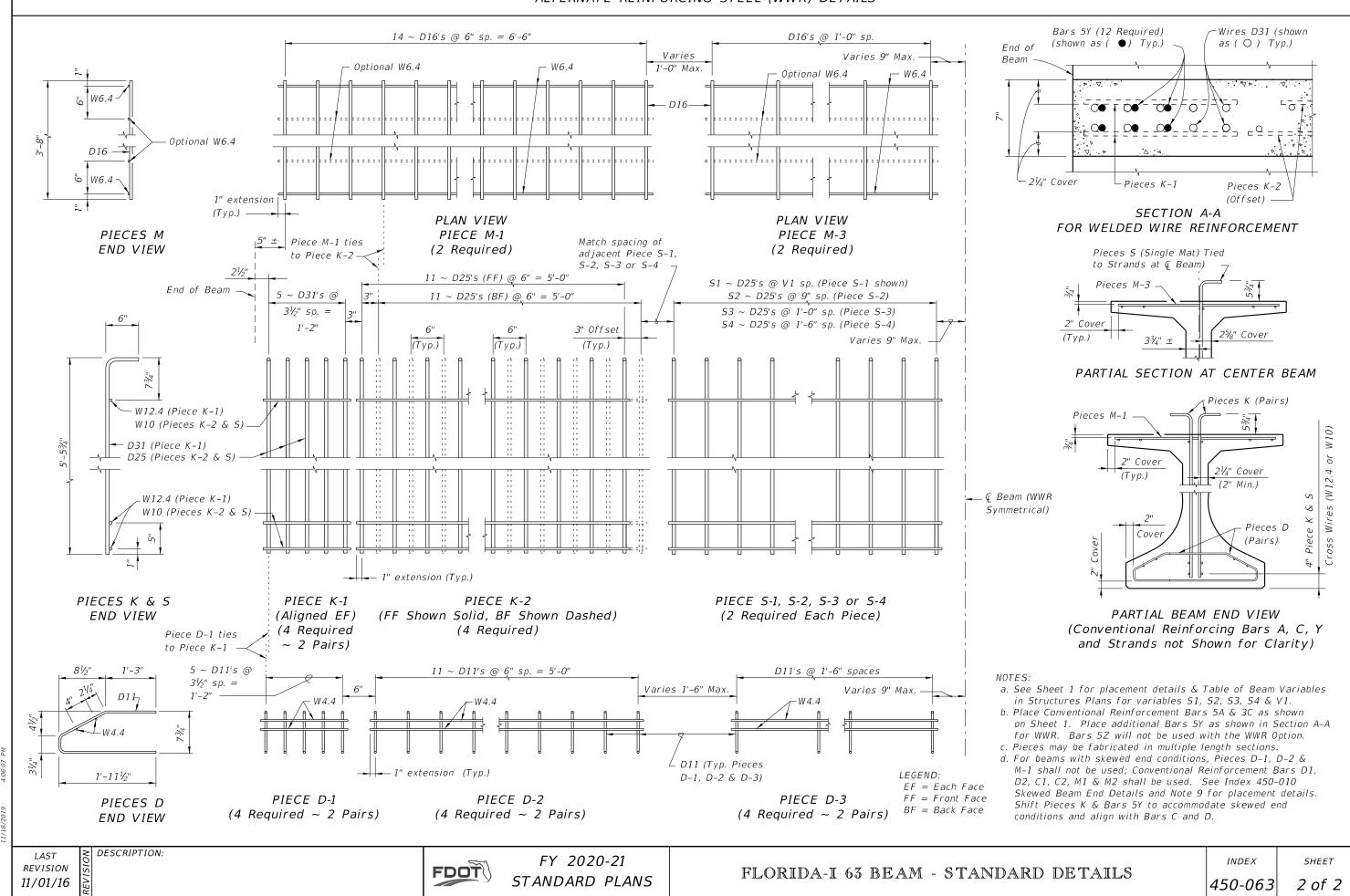


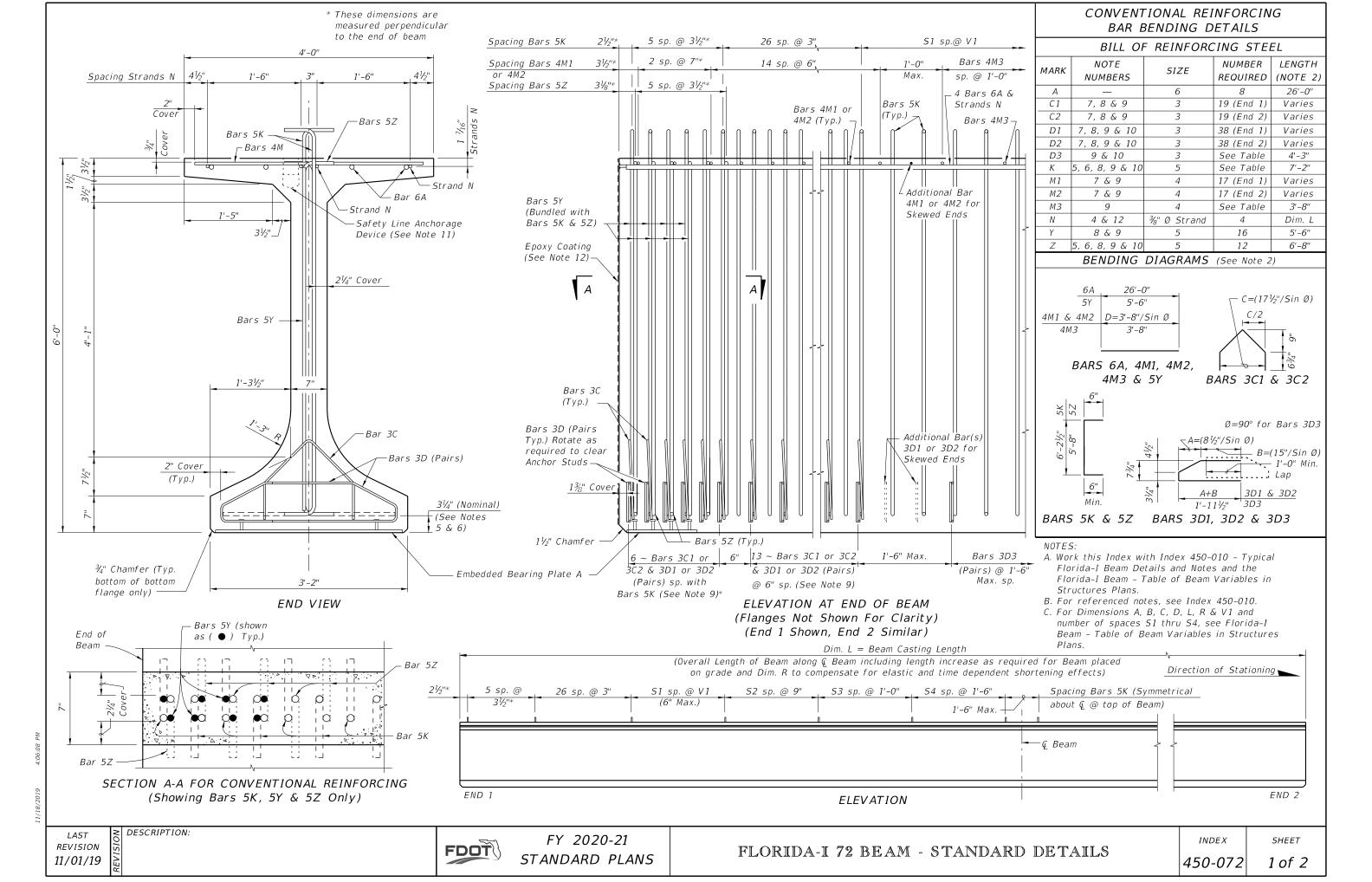


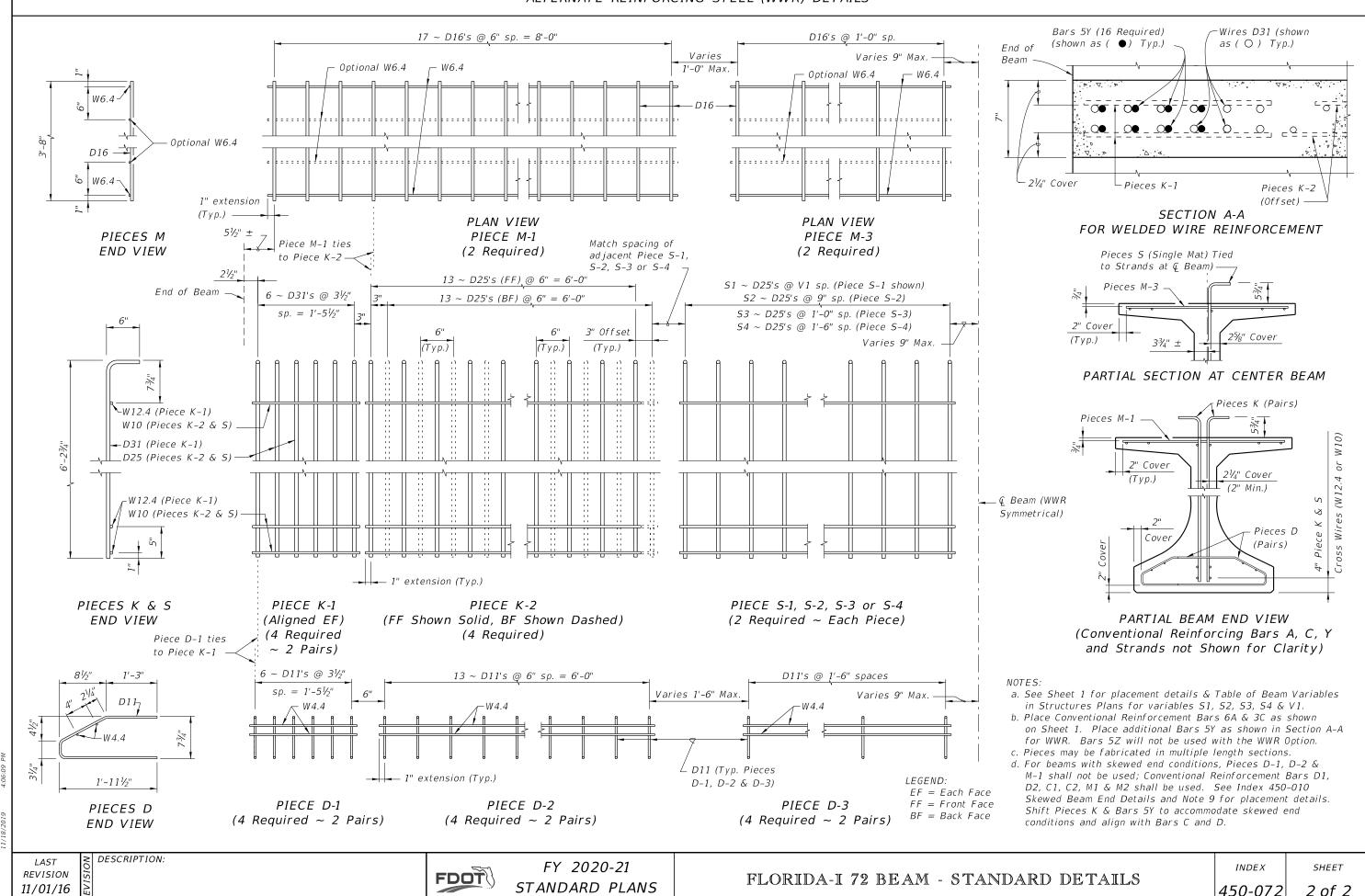


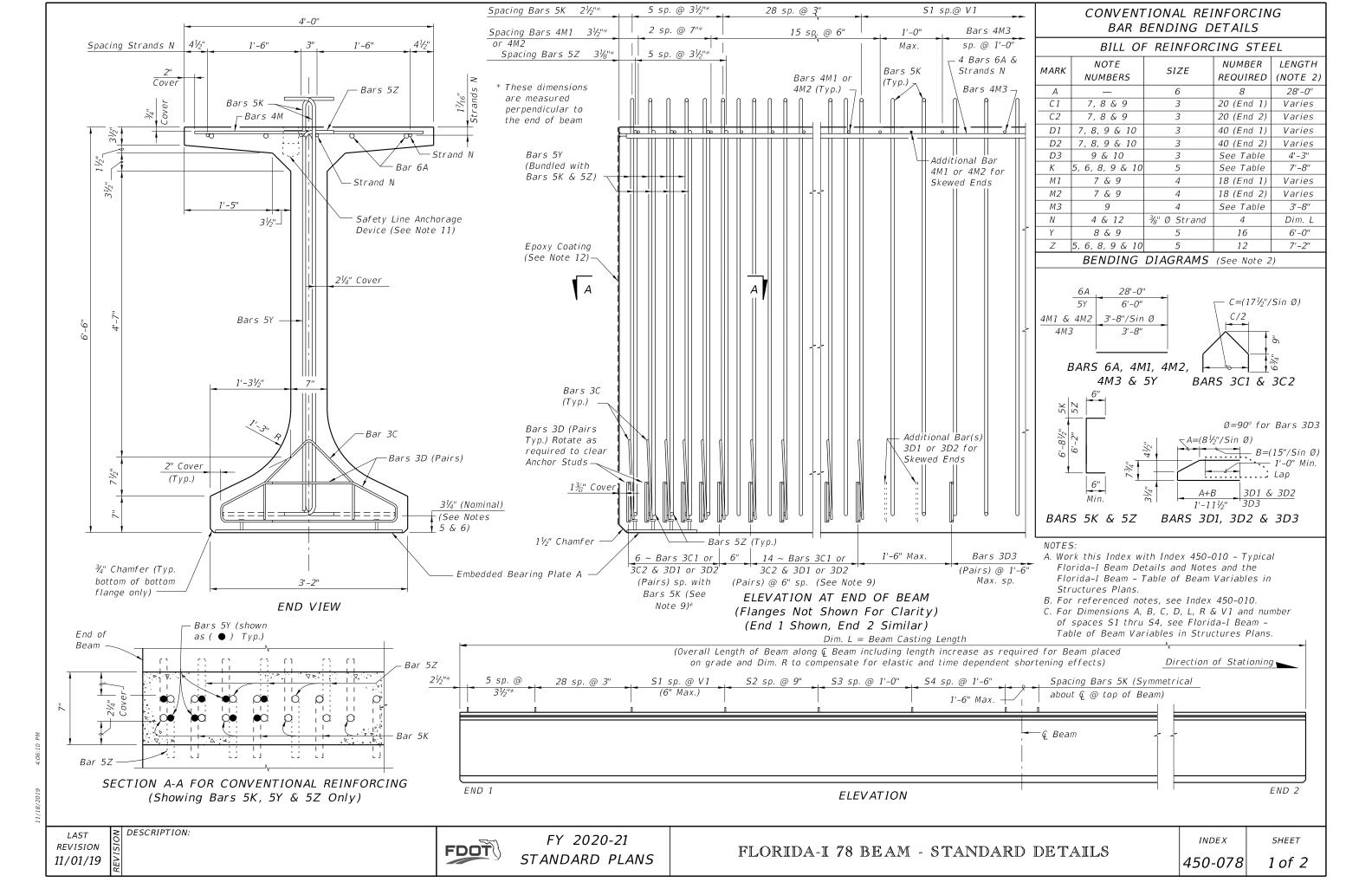


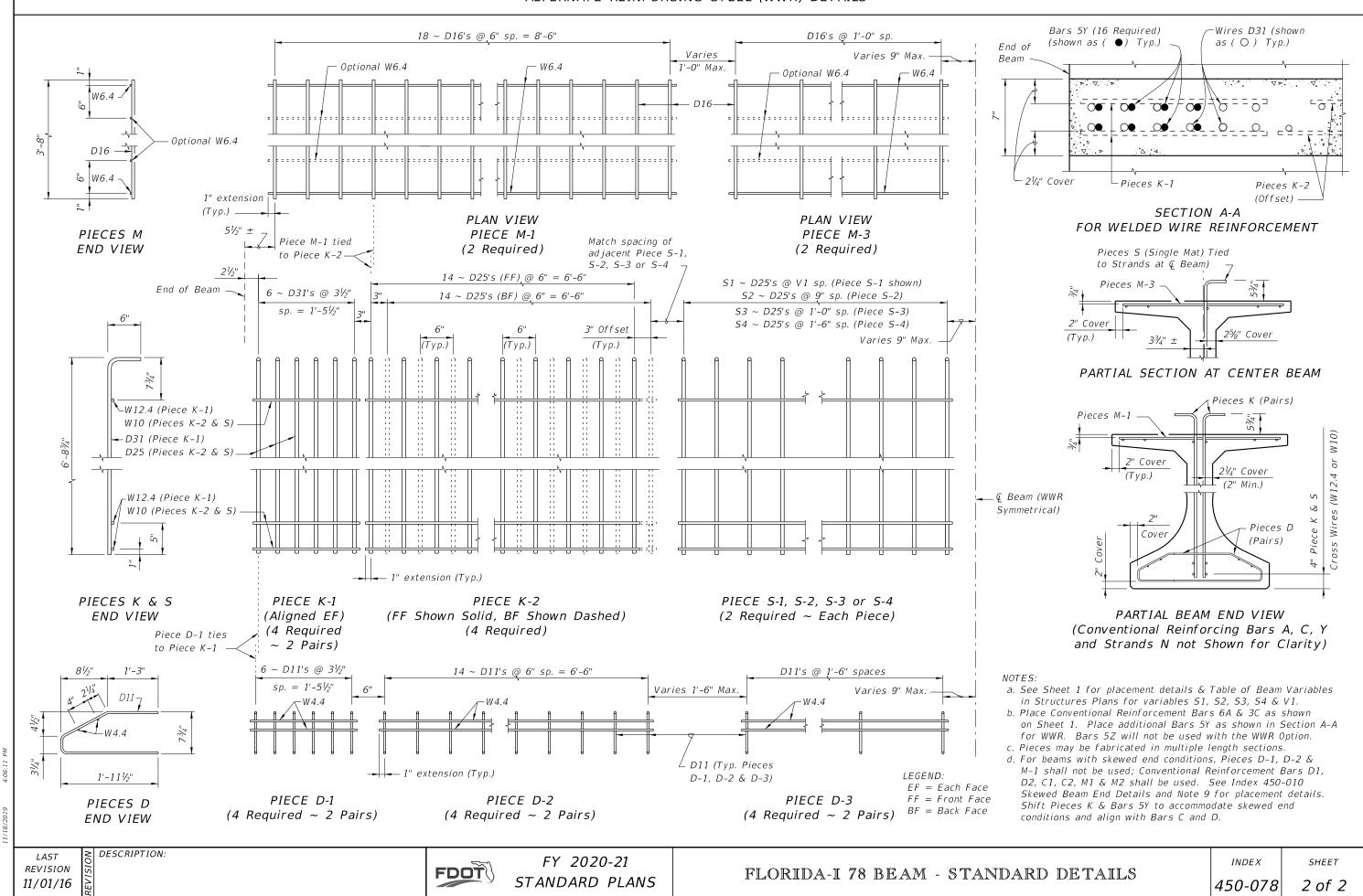


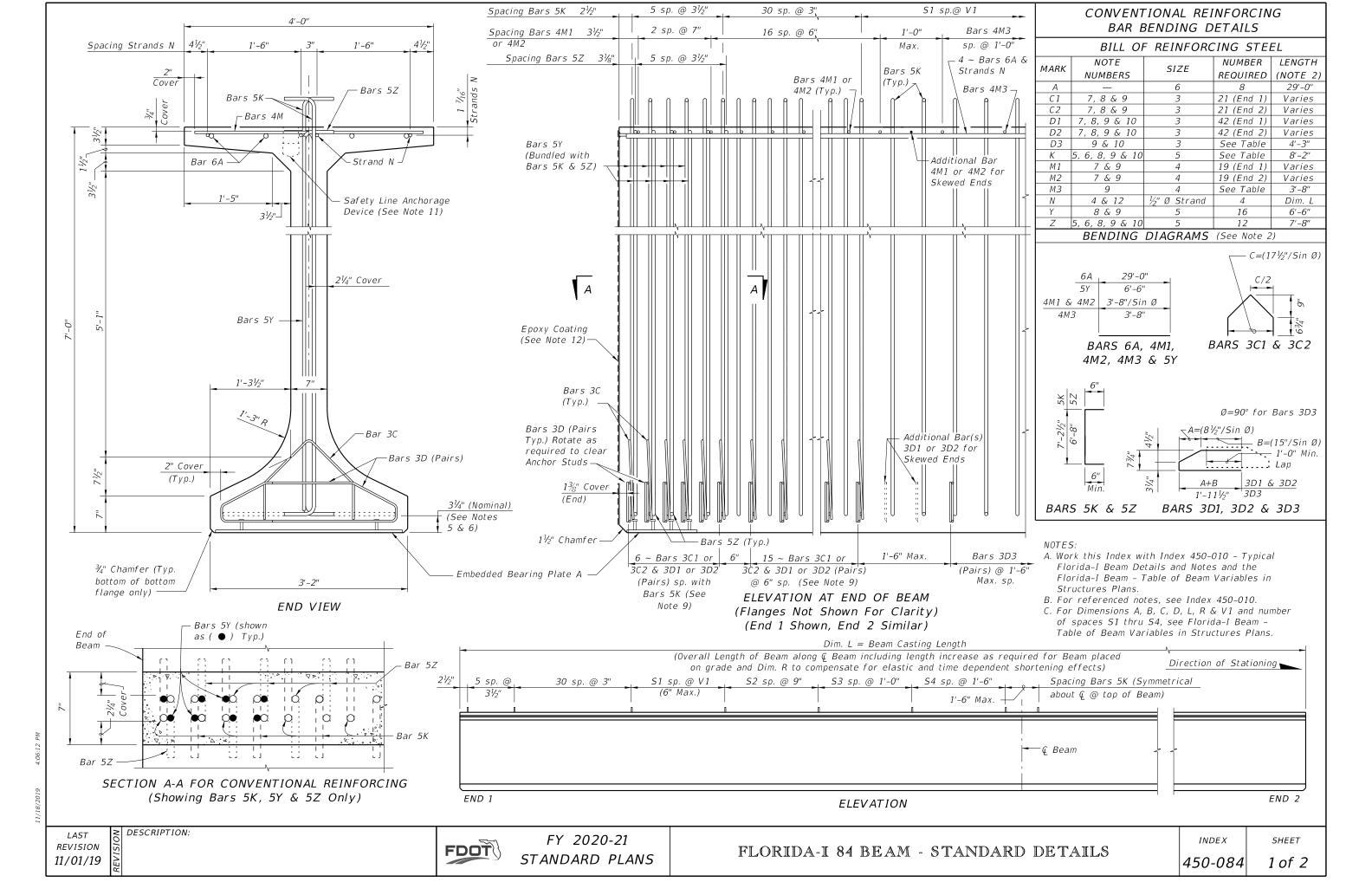


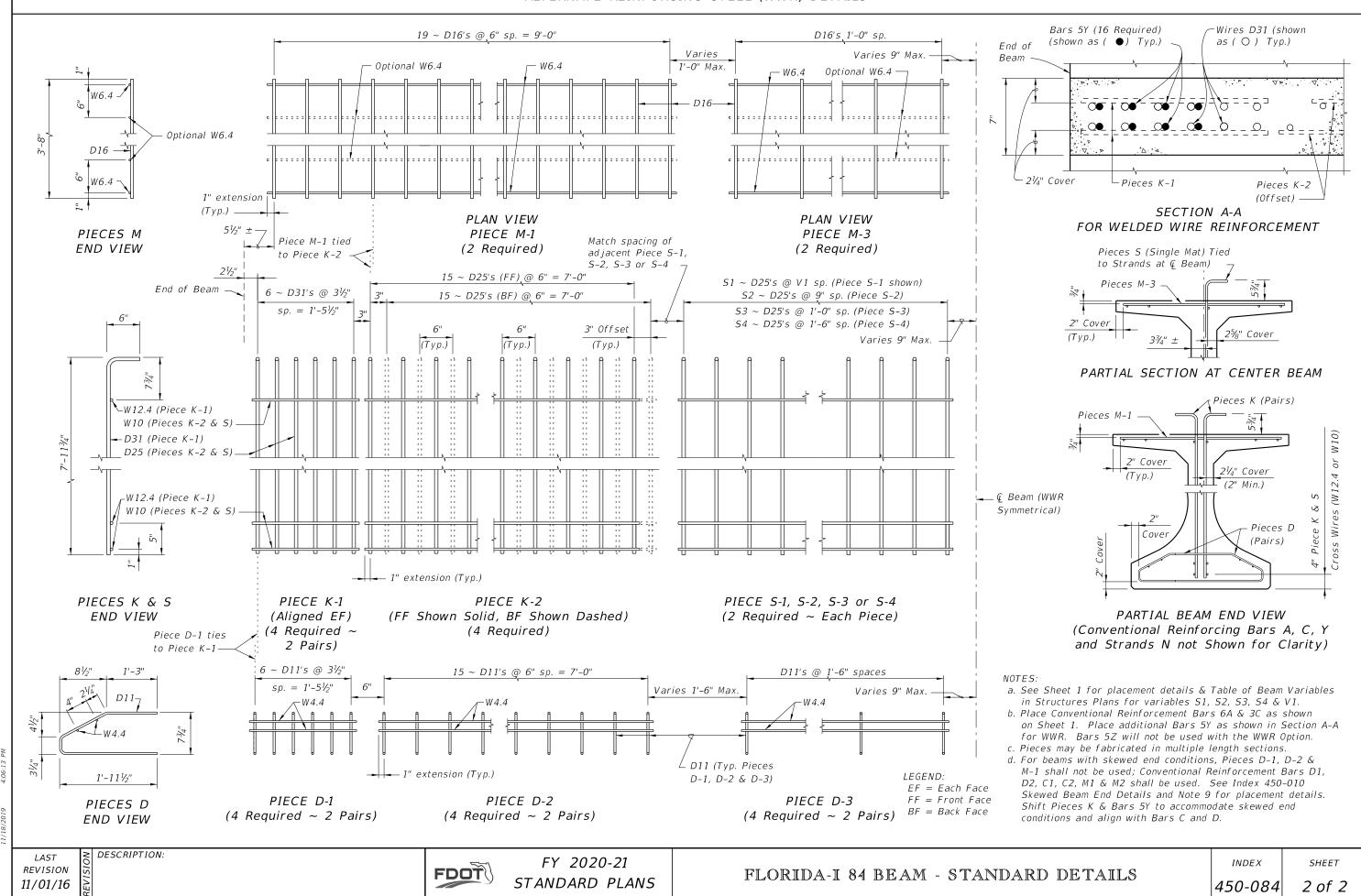


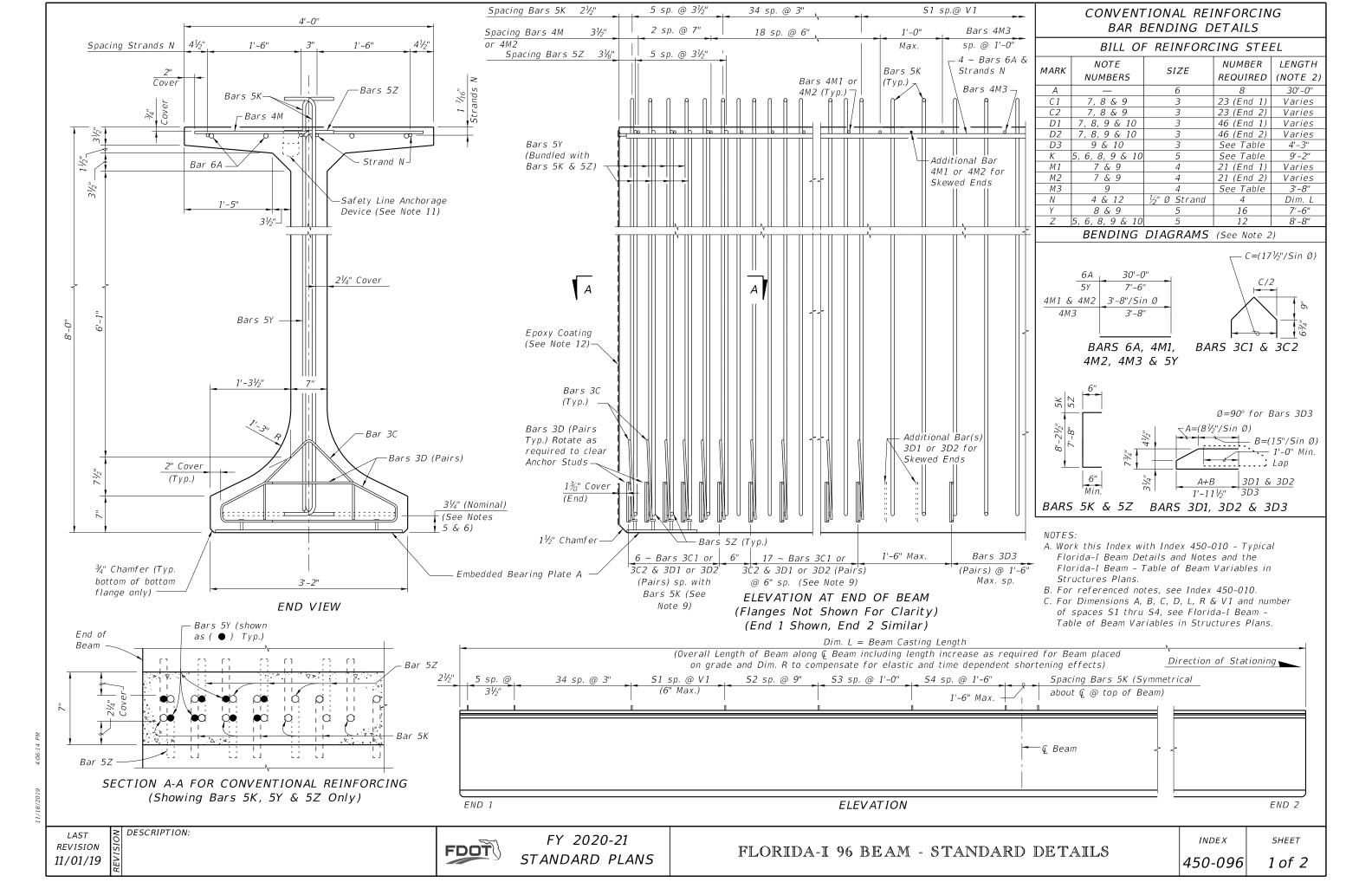


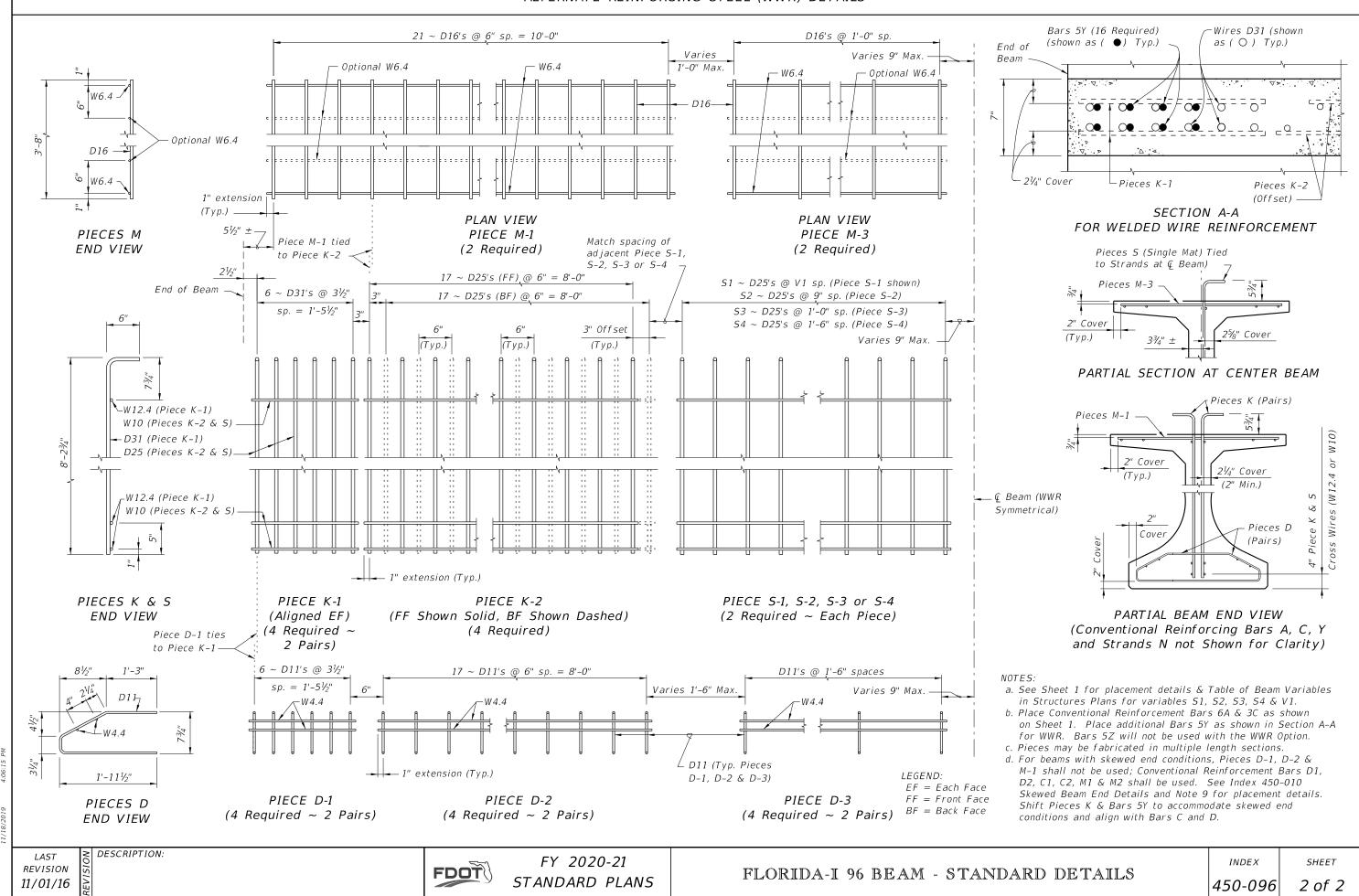


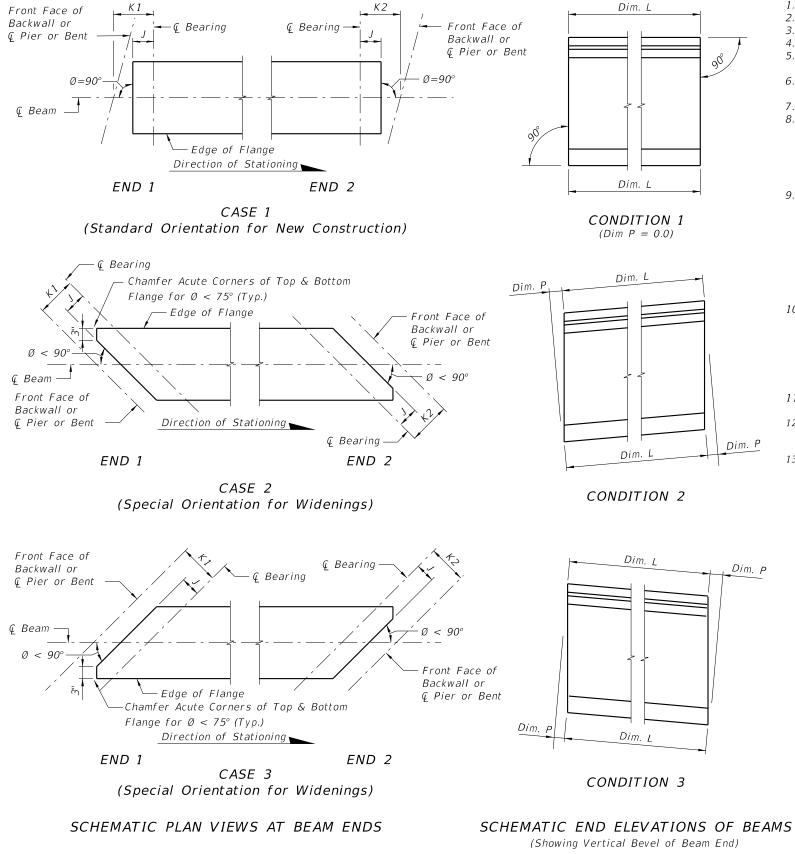












BEAM NOTES

- 1. Work this Index with the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out to out.
- 3. Concrete cover: 2 inches minimum.
 - 4. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
 - 5. Place one (1) Bar 4K or 5Z at each location. Alternate the direction of the ends for each
 - 6. Tie Bars 4K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
 - 7. Place Bars 3D1 in beam END 1, and Bars 3D2 in beam END 2.
 - 8. For Beams with vertically beveled end conditions:
 - A. Place first row of Bars 3D1, 3D2, 4K, 4Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1".
 - For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to minimum 1".
 - 9. For beams with skewed end conditions:
 - WWR is not permitted for end reinforcement Bars 3D1, and 3D2 on skewed ends; use bar reinforcement.
 - Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3D1, 3D2, 4K, 4Y and 5Z placed within the limits of the spacing for Bars 3D in "ELEVATION AT END OF BEAM".
 - Beyond the limits of the spacing for Bars 3D, place Bars 4K perpendicular to the longitudinal axis of the beam. For placement see "SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES" (Sheet 2).
 - 10. Contractor Options:
 - A. Deformed WWR may be used in lieu of Bars 3D, 4K, and 5Z as shown on Sheet 4; except at skewed ends (See Note 9).
 - Bars 3D1 and 3D2 may be fabricated as a two-piece bar with a 1'-0" minimum lap splice of the bottom legs.
 - For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces K & S under the cross wires on the bottom row of strands or above Strands N.
 - 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of required anchorage devices.
 - 12. For beams with ends that will not to be encased in concrete diaphragms, cut wedges and recess Prestressing Strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2.
 - 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
 - The superstructure environmental classification is slightly or moderately aggressive
 - Clear cover to adjacent steel reinforcing is 1"or greater
 - Hole inside diameter is 2" maximum
 - Non-metallic, non-water absorbing forming materials such as PVC, may be left in place permanently.

DETAILS AND NOTES

REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

AASHTO TYPE II BEAM

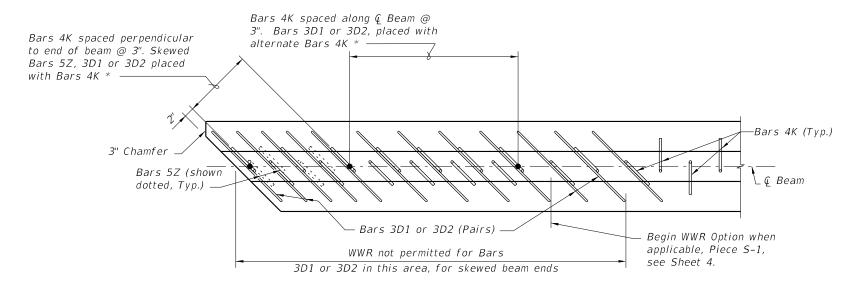
INDEX

SHEET

450-120 1 of 4

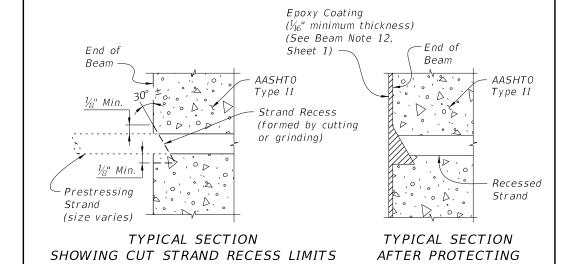
PARTIAL PLAN VIEW (SHOWING TOP FLANGE) (End 1 Shown, End 2 Similar) (Bars 5A, 4Y & Strands N not shown for clarity)

* For number of Bars, spacing and placement details see Sheet 3. See Sheet 3 for Conventional Reinforcement, Sheet 4 for WWR.



PARTIAL SECTION THRU WEB (SHOWING BOTTOM FLANGE) (End 1 Shown, End 2 Similar) (Bars 4Y & Strands not shown for clarity)

= SKEWED BEAM END DETAILS FOR WIDENING EXISTING BRIDGES =



=== STRAND CUTTING AND PROTECTING DETAIL ====

DETAILS AND NOTES

REVISION 11/01/19

DESCRIPTION:

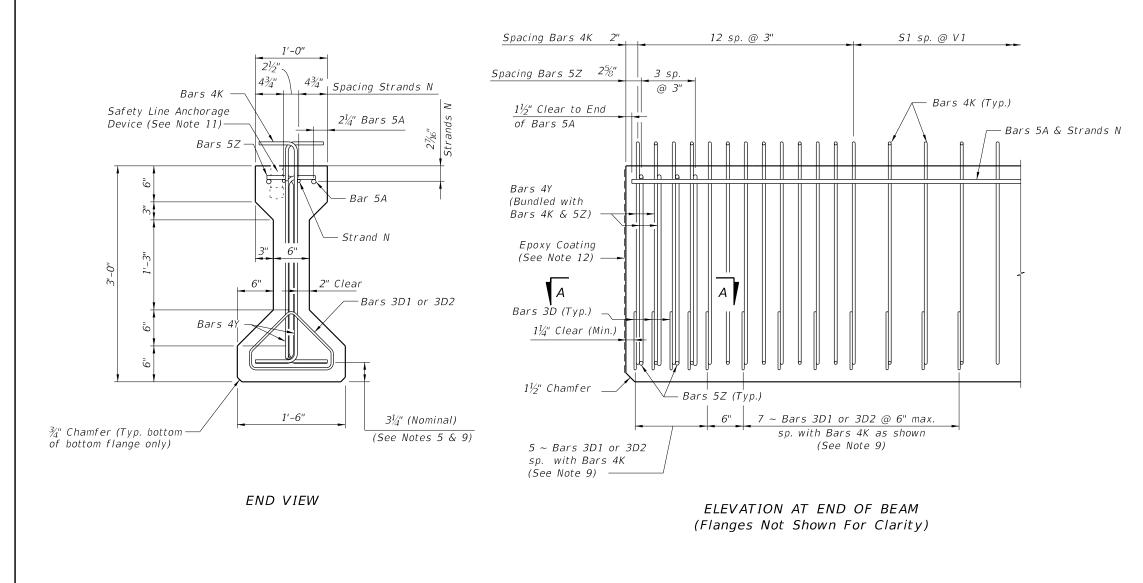
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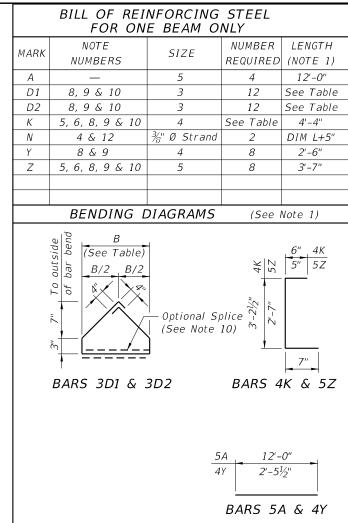
FY 2020-21 STANDARD PLANS

AASHTO TYPE II BEAM

INDEX 450-120

SHEET



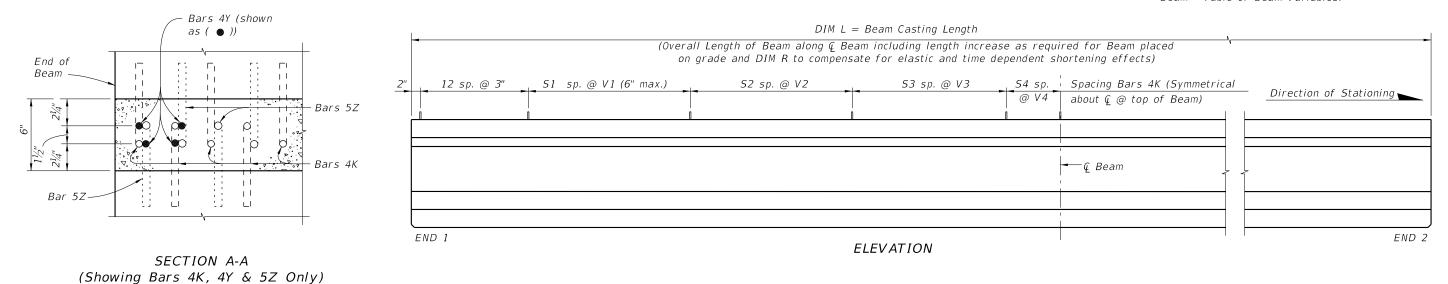


NOTES

Work this Index with the AASHTO Type II Beam -Table of Beam Variables in Structures Plans.

For referenced notes, see Sheet 1.

For Dimensions L, R, V1 thru V4 and number of spaces S1 thru S4, see AASHTO Type II Beam - Table of Beam Variables.



11/18/2019

LAST O DESCRIPTION:
REVISION II/01/19

FDOT

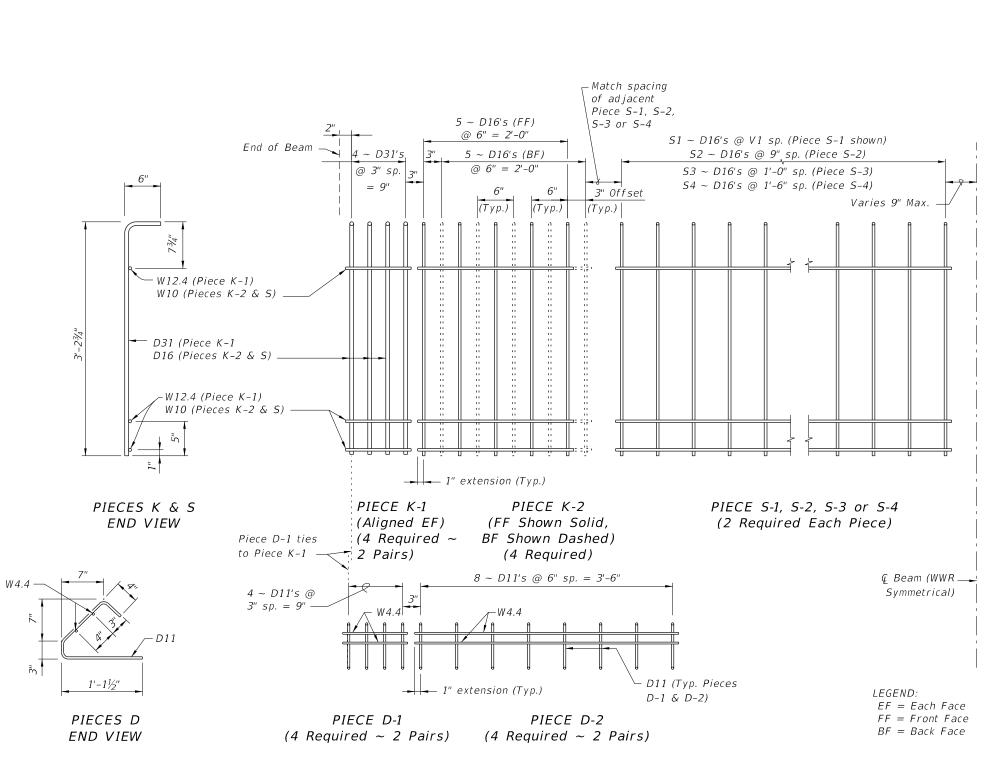
FY 2020-21 STANDARD PLANS

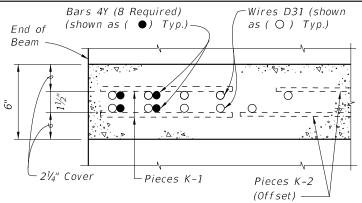
AASHTO TYPE II BEAM

INDEX 450-120

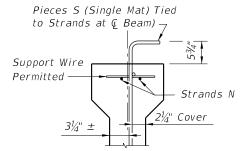
STANDARD DETAILS

3 of 4

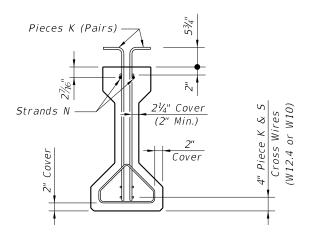




SECTION A-A FOR WELDED WIRE REINFORCEMENT



PARTIAL SECTION AT CENTER BEAM



PARTIAL BEAM END VIEW (Conventional Reinforcing Bars A, Y and Bottom Strands not Shown for Clarity)

NOTES:

- a. See Sheet 3 for placement details & Table of Beam Variables in Structures Plans for variables S1, S2, S3, 54 & V1.
- b. Place Conventional Reinforcement Bars 5A as shown on Sheet 3. Place additional Bars 4Y as shown in Section A-A for WWR. Bars 5Z will not be used with the WWR Option.
- c. Pieces may be fabricated in multiple length sections.
- d. For beams with skewed end conditions, Pieces D-1 & D-2 shall not be used; Conventional Reinforcement Bars D1 & D2 shall be used. See Sheet 2 Skew Details and Sheet 1 Note 9 for placement details. Shift Pieces K & Bars 4Y to accommodate skewed end conditions and align with Bars D.

STANDARD DETAILS

REVISION 11/01/16

FDOT

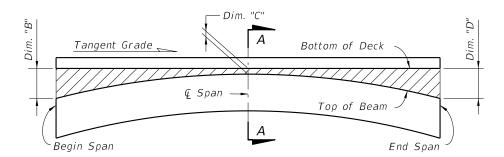
FY 2020-21 STANDARD PLANS

AASHTO TYPE II BEAM

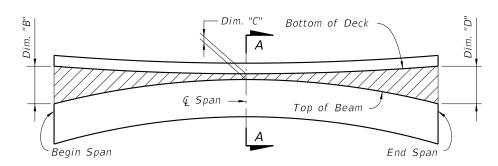
INDEX

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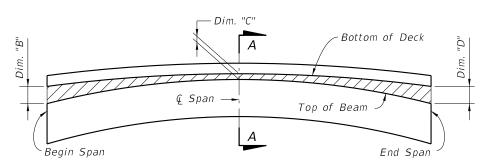
SHEET



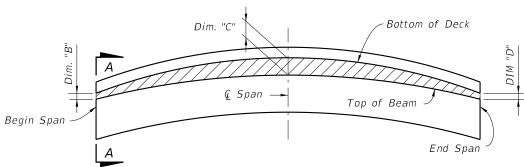
BUILD-UP DIAGRAM FOR TANGENT SPANS (ALONG G BEAM) (CASE 1)



BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q BEAM) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG Q BEAM) (CASE 3)

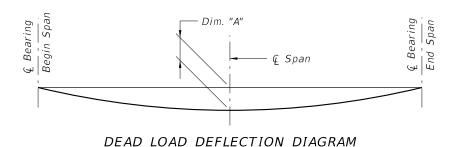


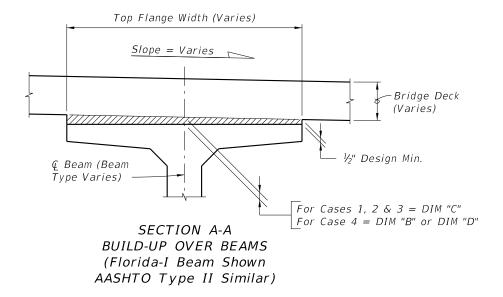
BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG Q BEAM) (CASE 4)

BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.





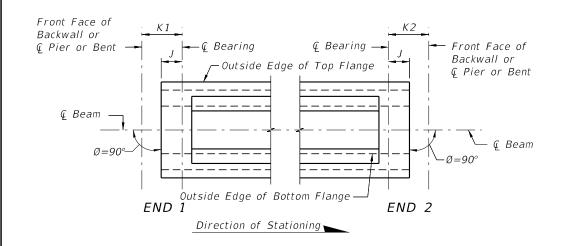
* NOTE:

Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.

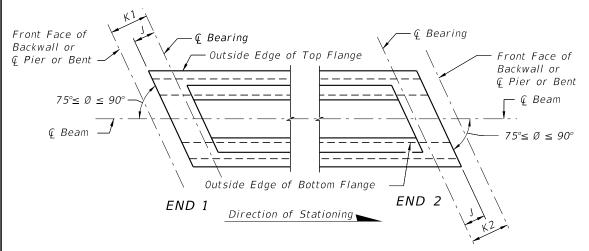
REVISION 07/01/15

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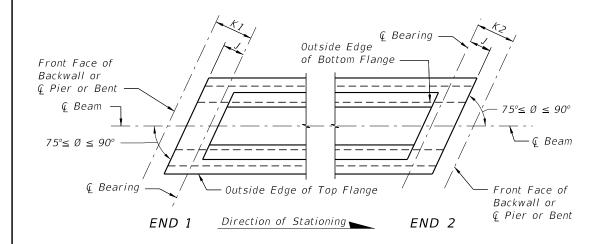
FDOT



CASE 1



CASE 2



CASE 3

SCHEMATIC PLAN VIEWS AT BEAM ENDS =

LAST ODESCRIPTION: REVISION IS 11/01/16

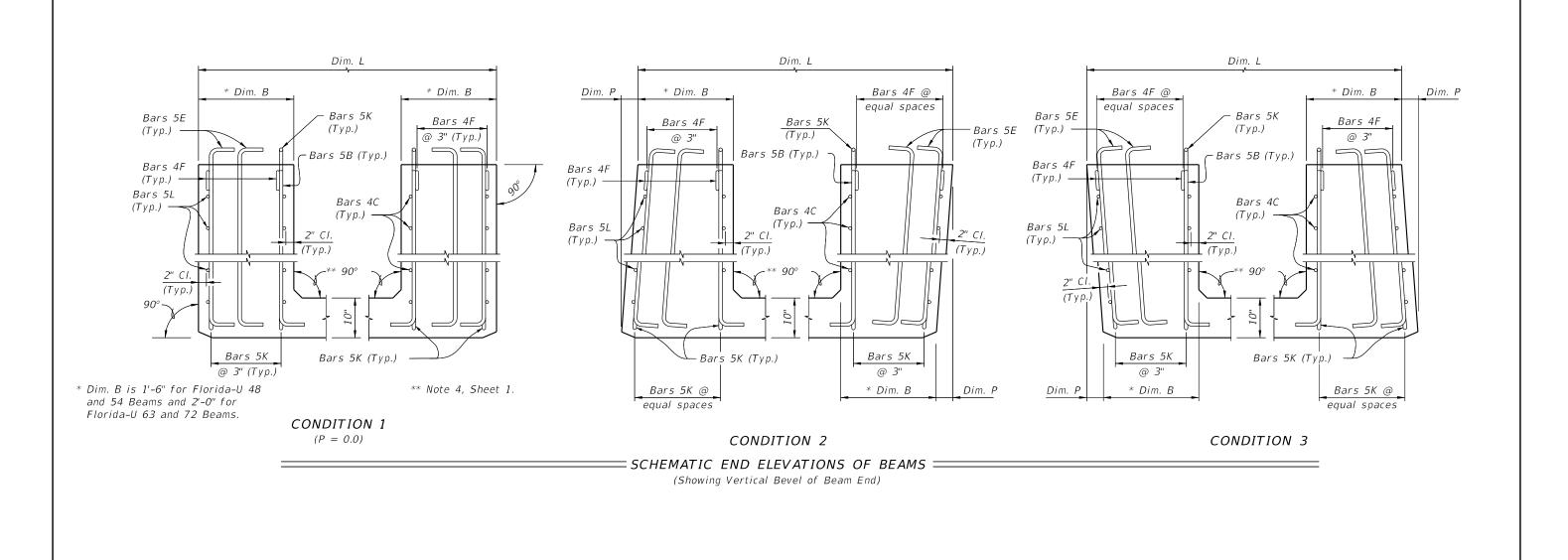


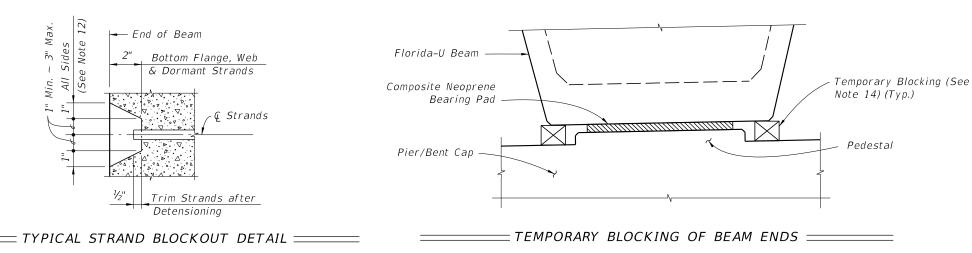
FY 2020-21 STANDARD PLANS

BEAM NOTES

- 1. Work this Index with the Florida-U Beam Standard Details (Index 450-248, 450-254, 450-263 and 450-272) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out.
- 3. Concrete cover: 2 inches minimum. Maximum aggregate size is a No. 67.
- 4. Concrete face may be sloped with a maximum 1:24 draft to facilitate formwork removal.
- 5. Strands N: 3/8" Ø minimum, stressed to 10,000 lbs. each.
- 6. Tie Bars 5K to the fully bonded strands in the bottom row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans).
- 7. For beams without skewed ends or vertically beveled end conditions (see Note 8) the Engineer may approve the use of deformed WWR in lieu of Bars 6A1, 4A2, 5B, 4C, 3D, 5E, 4F, 4G, 4H, 5K, 5L and 4M. The spacing and sizes of deformed WWR must match the reinforcing sizes shown on the Florida-U Beam Standard Details sheets.
- 8. For Beams with vertically beveled end conditions, where "Dim. P" exceeds 1", place Bars 5E, and the first Bars 4F and 5K parallel to the end of the beam. Fan the remaining Bars 4F and 5K within the limits of "Dim. B" (End Diaphragm) at equal spaces until vertical.
- 9. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any anchorage devices or other required embedded hardware.
- 10. Intermediate diaphragms must be cast and concrete release strength obtained prior to removing the beam from casting bed.
- 11. Place drains pipes adjacent to each web at each beam end (four drains per beam).

 A. Drain Pipe: 2" NPS Schedule 80 PVC.
 - B. Cover, wrap and secure wire screen around the end of the pipe prior to casting. Extend screen a minimum of 1" down the pipe sides.
 - C. Provide removable pipe plugs during casting. Remove plugs from the inside of pipes after casting.
- 12. Protection of Strands:
 - A. Provide a 2" deep recess around all strands (including dormant) or strand groups. Extend the recessed blockout to the web face and bottom of the flange for the bottom row of strands.
 - B. After detensioning, cut strands ½" from recessed surface and fill the blockout to protect strands with Type F-2 or Q Epoxy Compound in accordance with Specification Section 926.
- 13. Use Stay-In-Place metal deck forms inside the beams.
- 14. Prior to deck placement, provide temporary blocking under each web at both ends of every beam. Ensure the temporary blocking is adequate to resist movements and rotations during deck placement. Leave temporary blocking and bracing in place for a minimum of four days after the deck is placed.
- 15. Based on the deck forming system and deck placement sequence, evaluate and provide any required temporary bracing between the U Beams.





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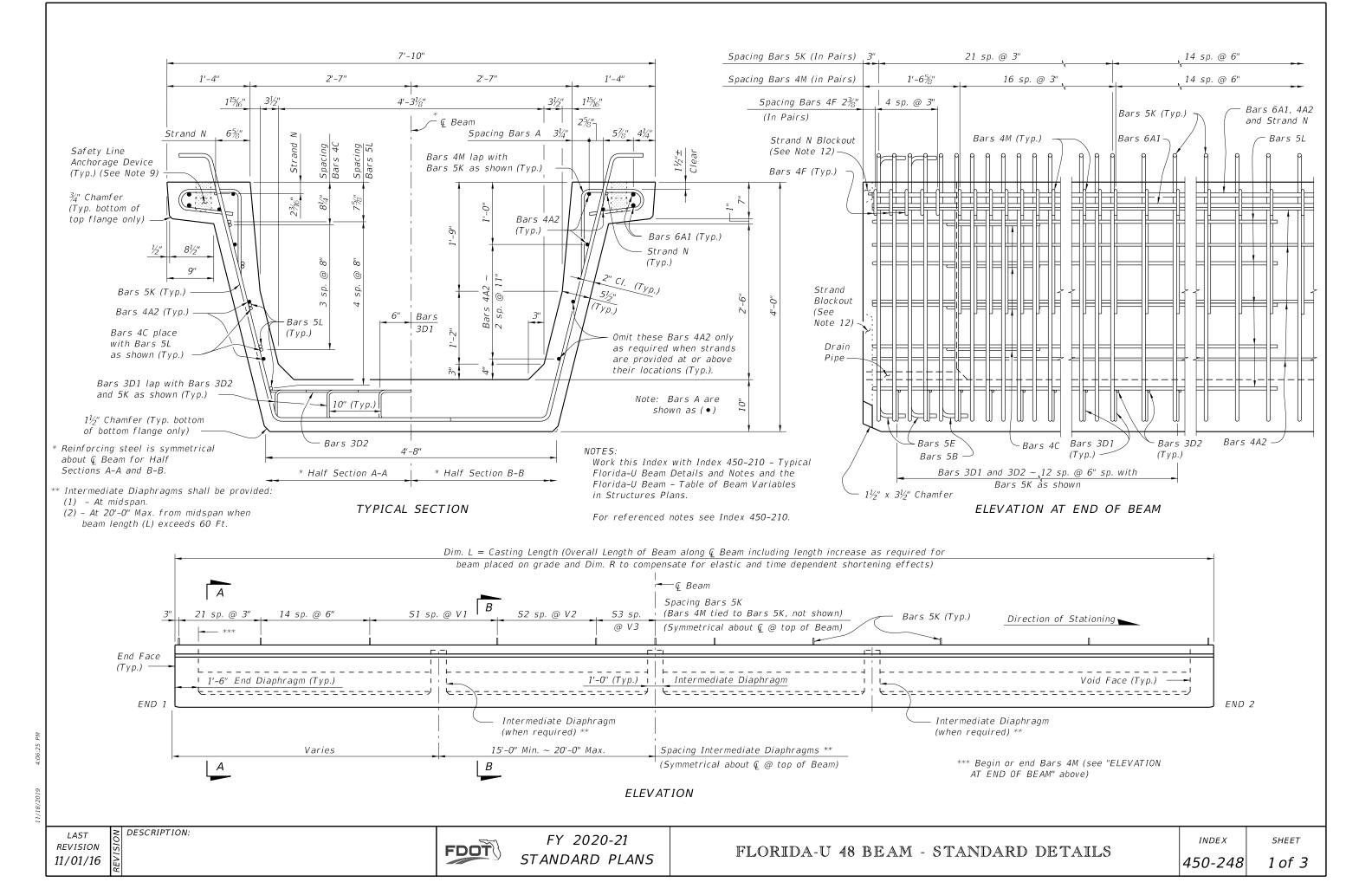
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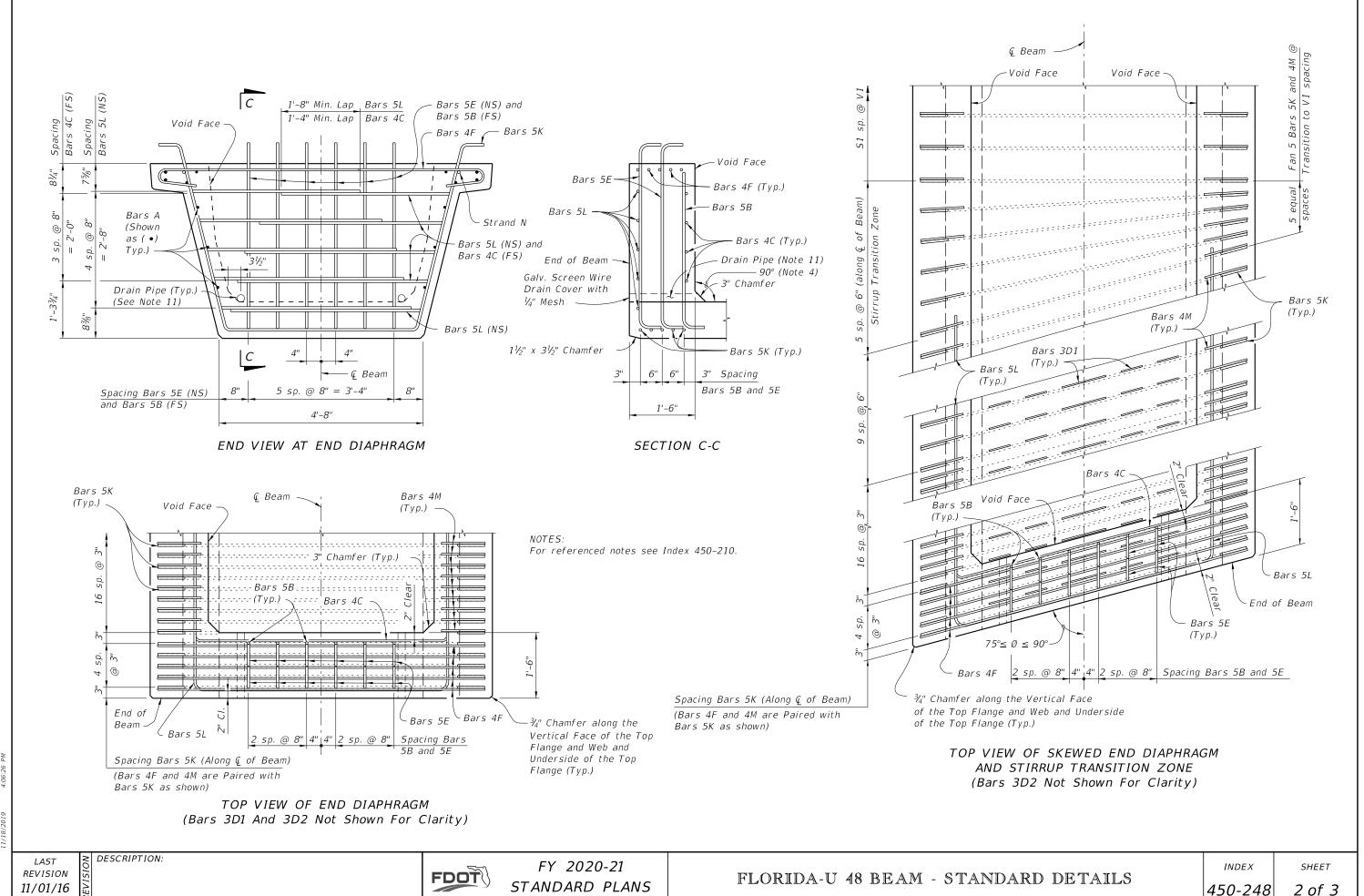
FY 2020-21 STANDARD PLANS

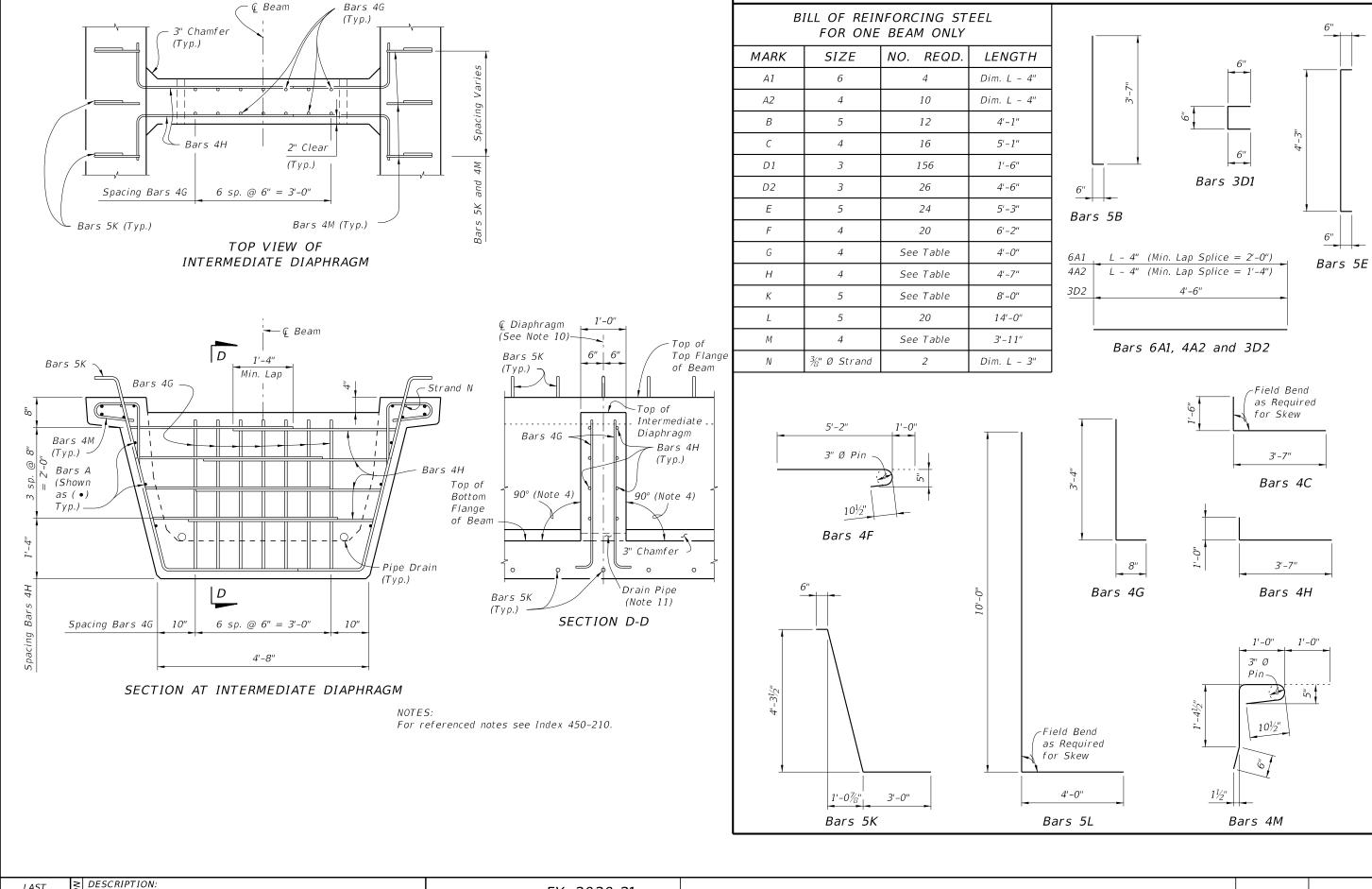
FLORIDA-U BEAM - TYPICAL DETAILS & NOTES INDEX

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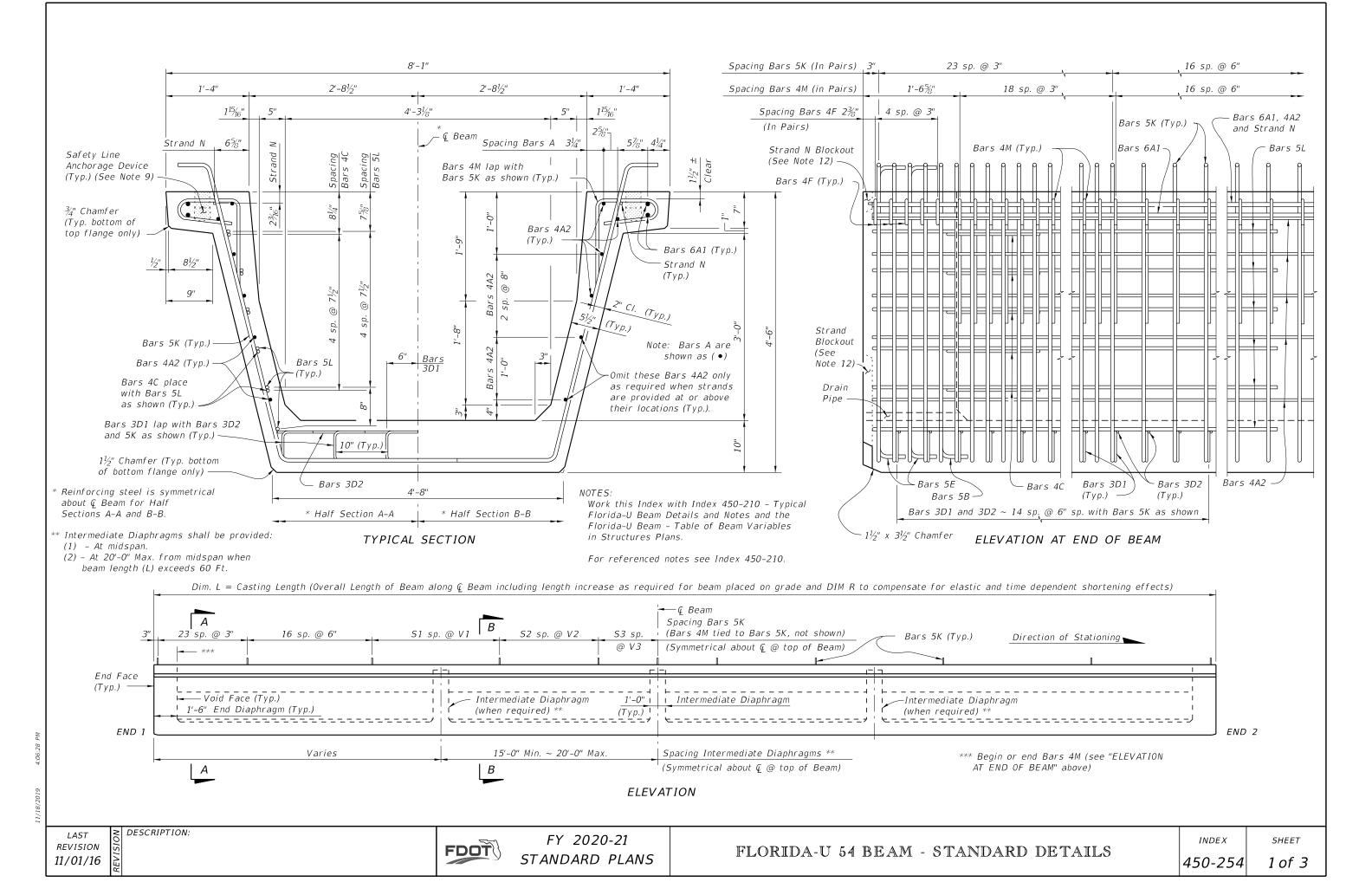
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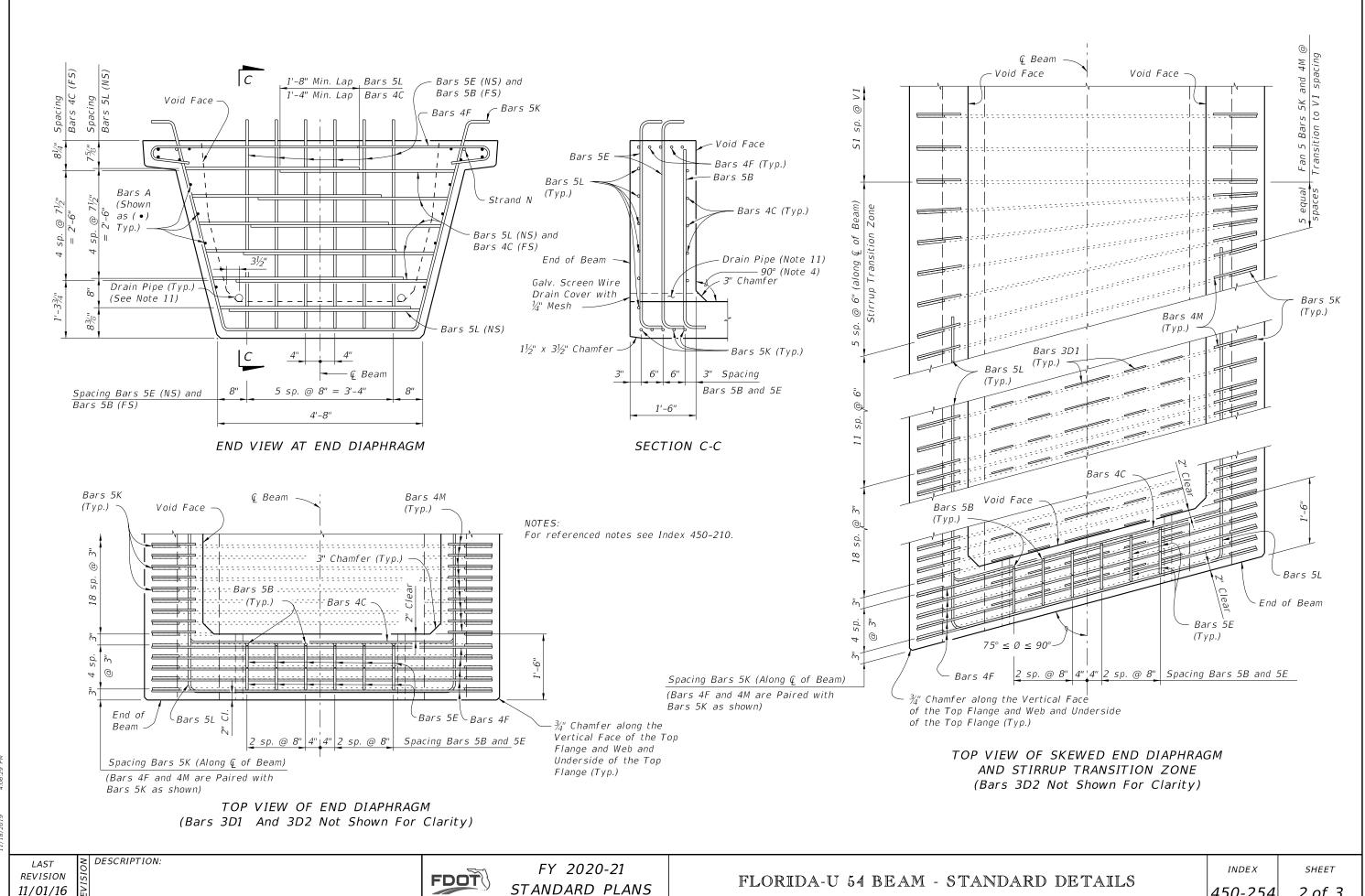
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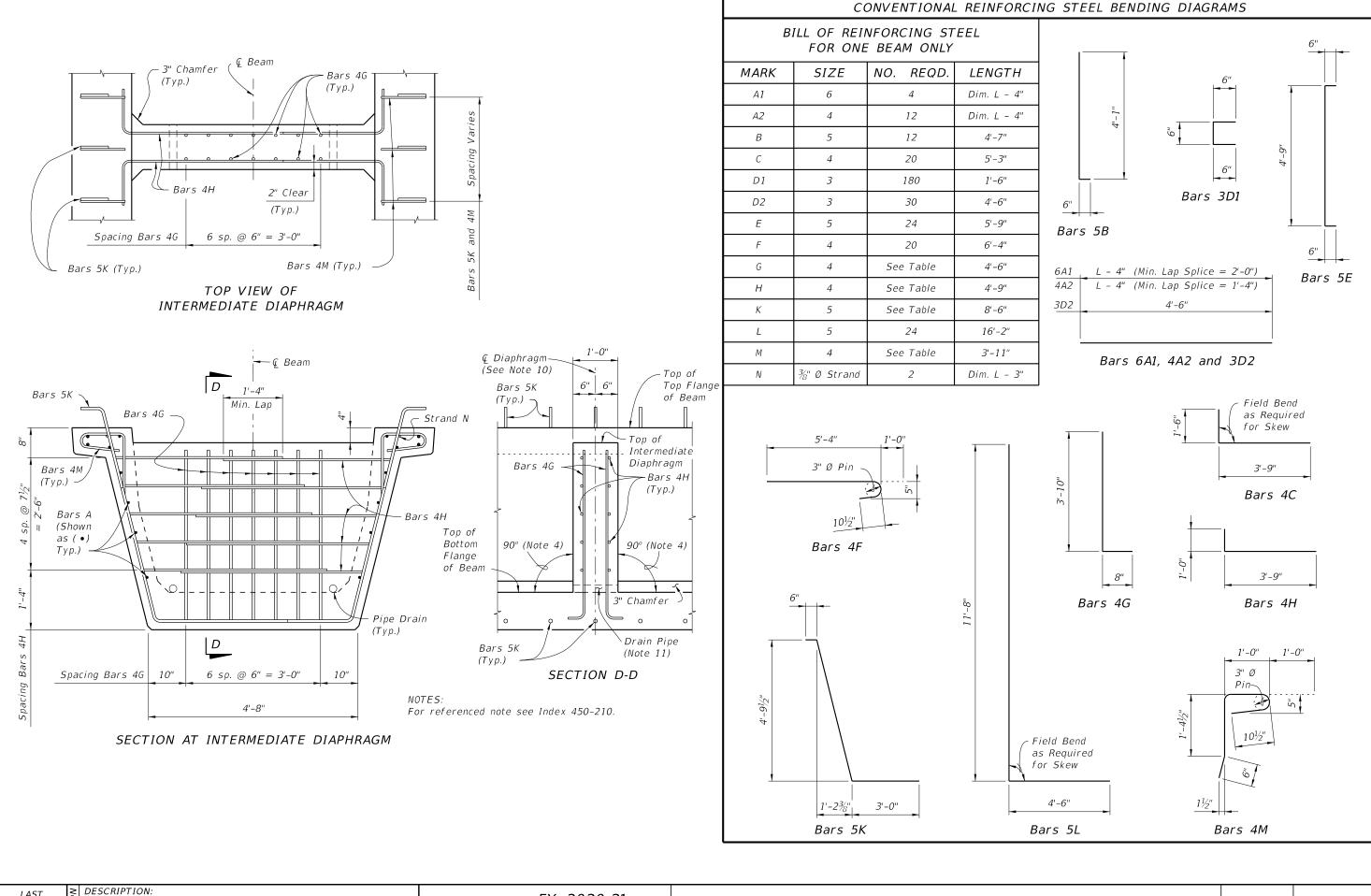
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CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

SHEET



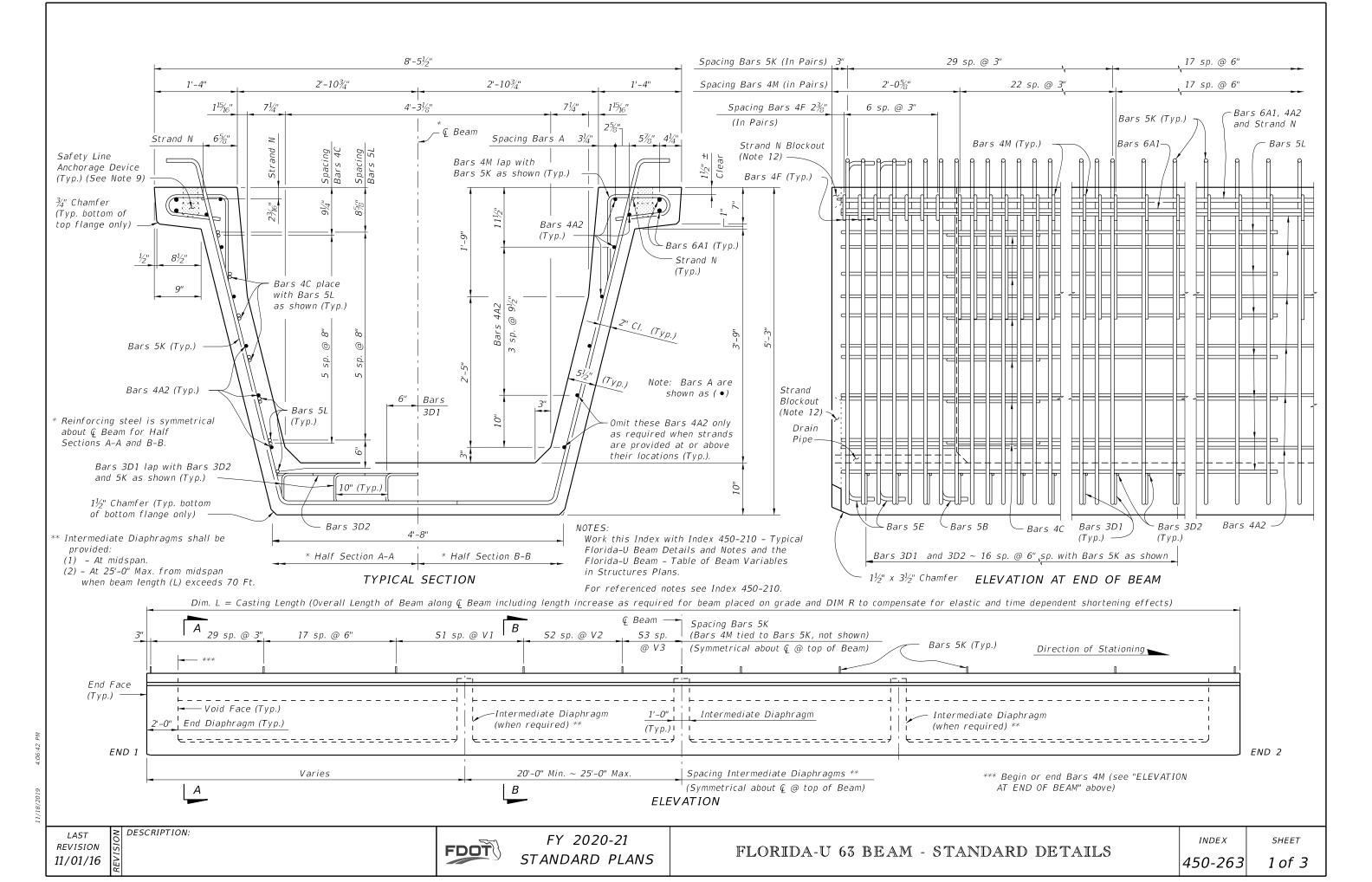


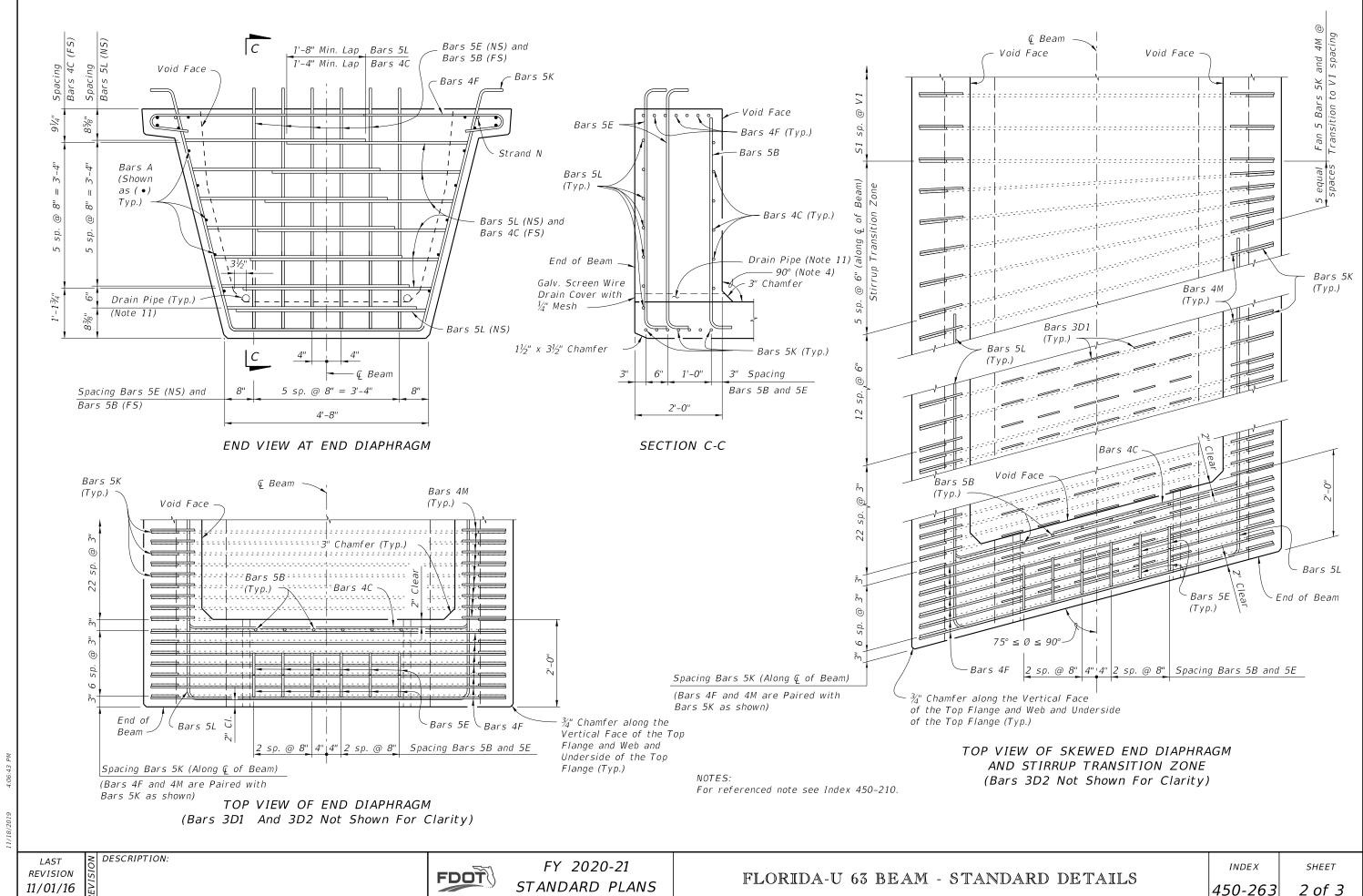


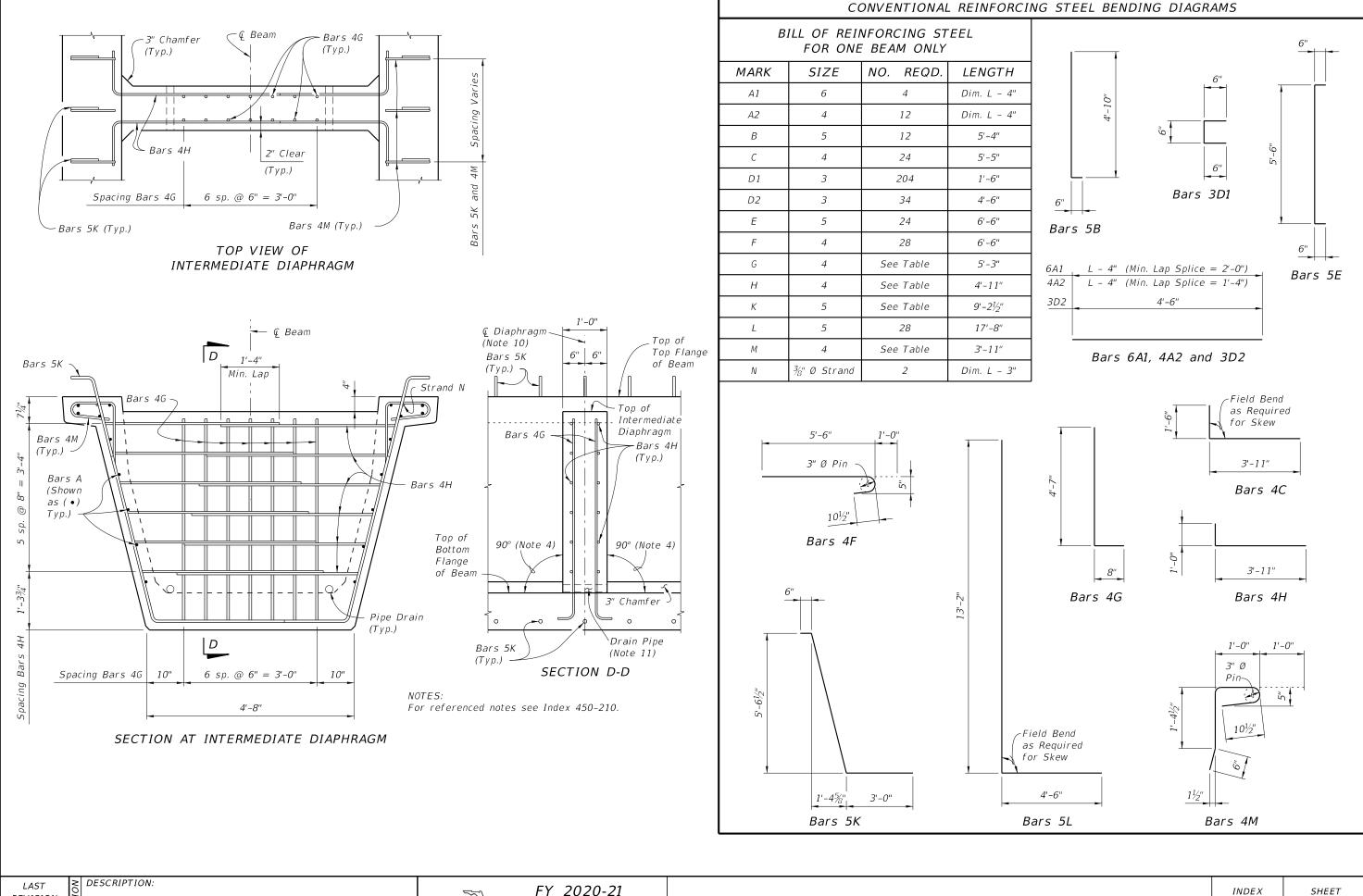
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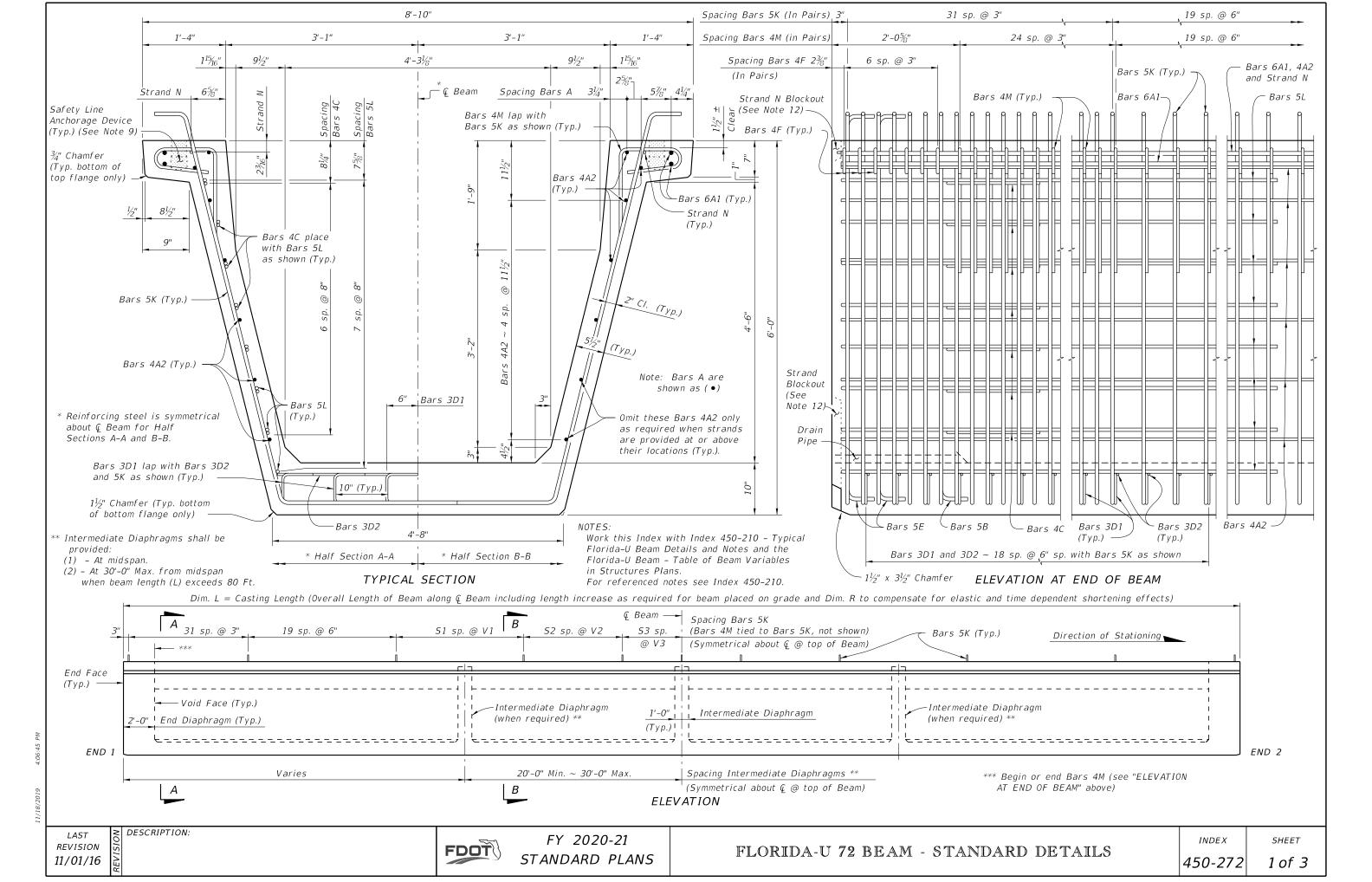


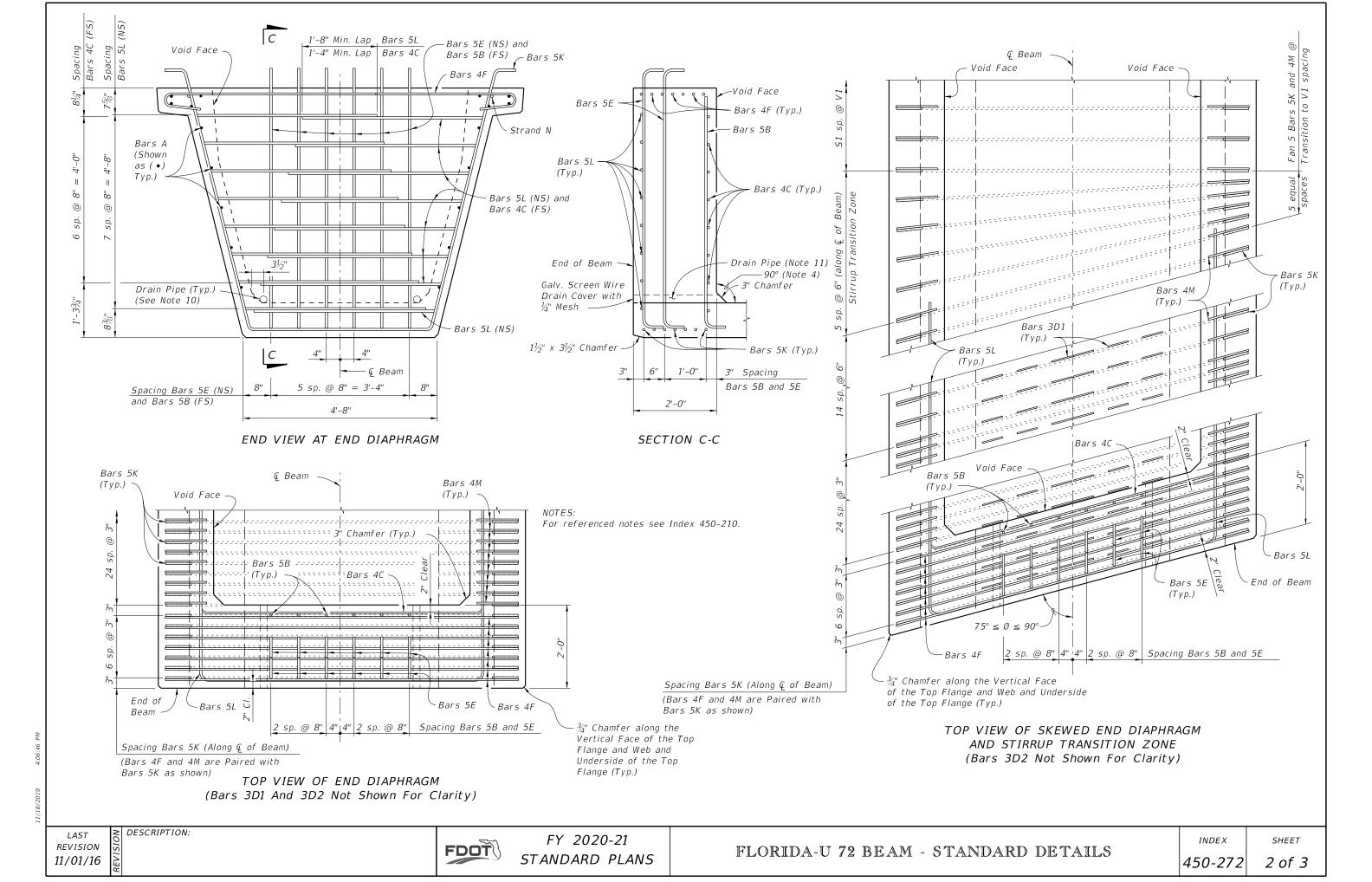
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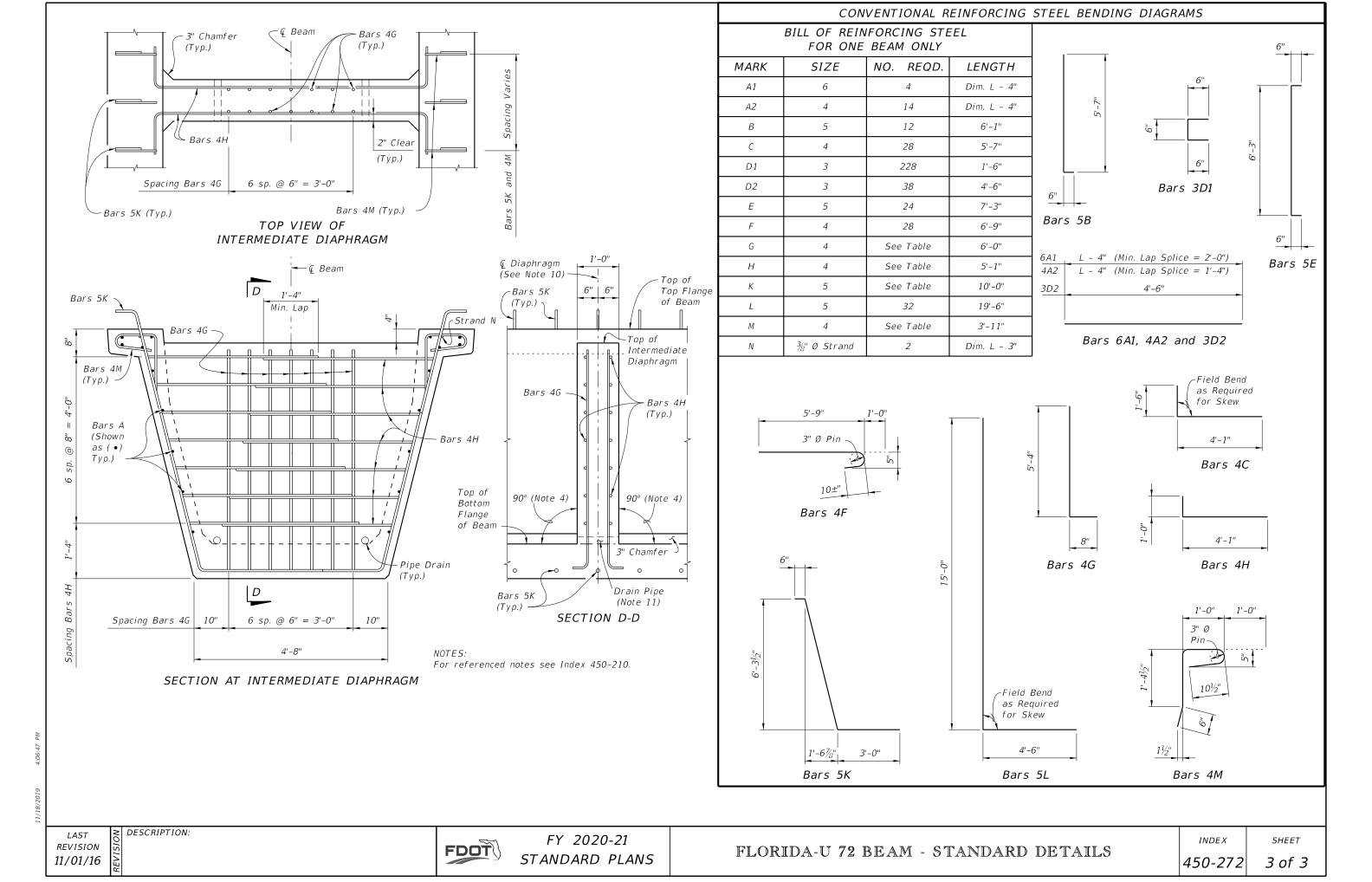
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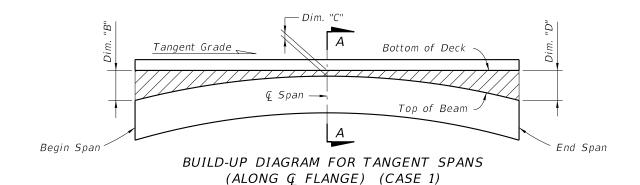
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FY 2020-21 STANDARD PLANS



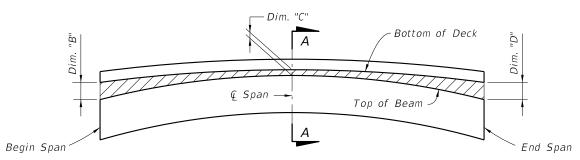




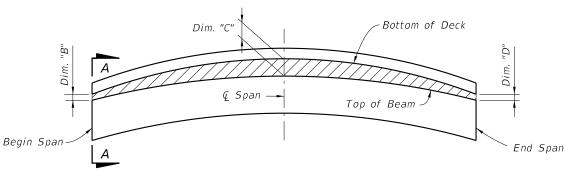


Bottom of Deck Α Top of Beam End Span Begin Span

BUILD-UP DIAGRAM FOR SAG VERTICAL CURVE & HORIZONTAL CURVE SPANS (ALONG Q FLANGE) (CASE 2)



BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT Q SPAN (ALONG G FLANGE) (CASE 3)

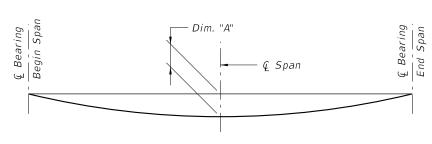


BUILD-UP DIAGRAM FOR CREST VERTICAL CURVE SPANS - CONTROL AT BEGIN OR END SPAN (ALONG G FLANGE) (CASE 4)

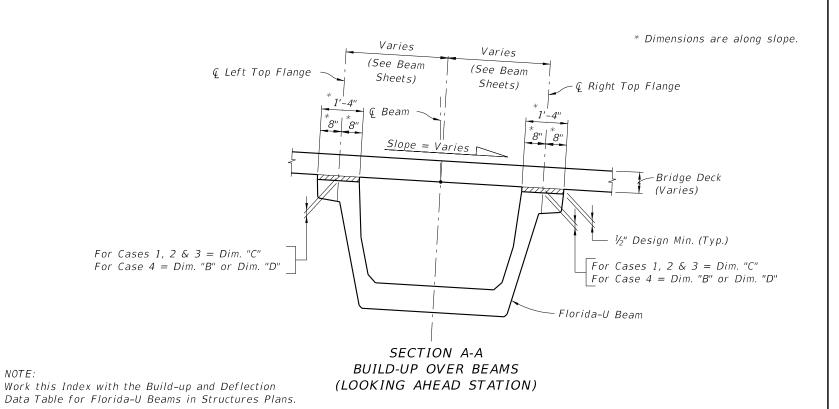
BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1/2" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.



DEAD LOAD DEFLECTION DIAGRAM (ALONG G BEAM)



DESCRIPTION: **REVISION** 07/01/15

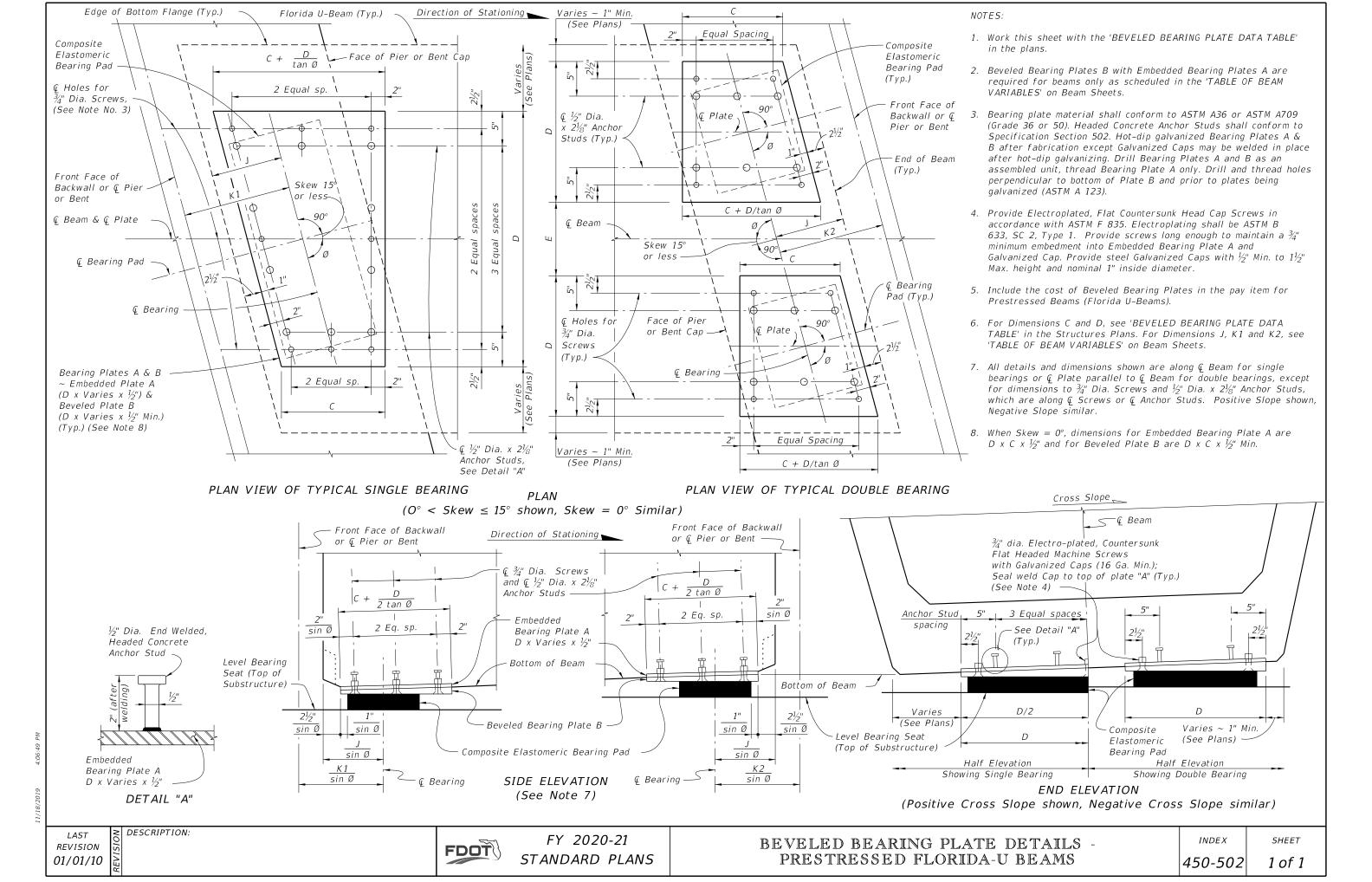
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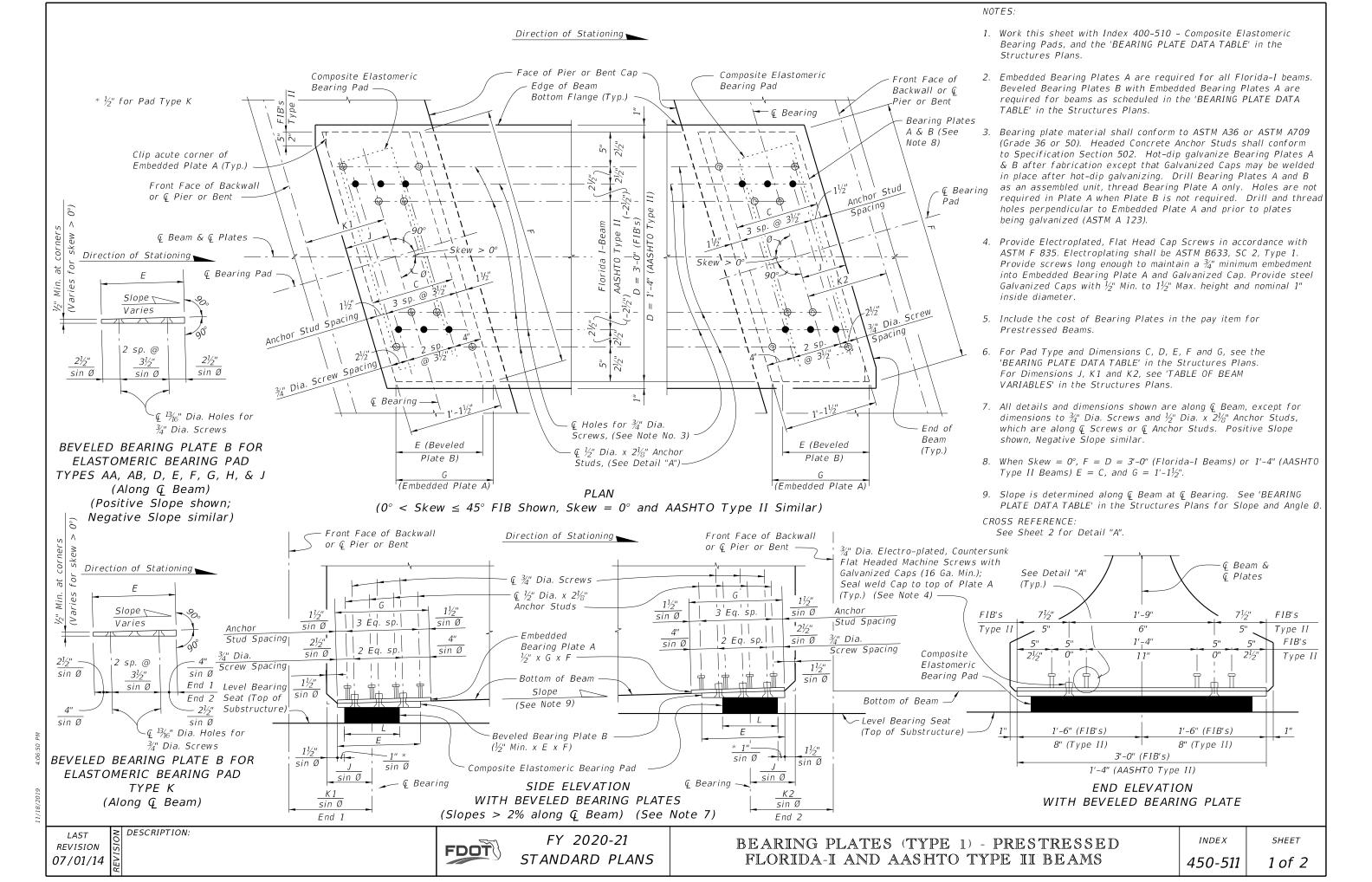
FY 2020-21 STANDARD PLANS

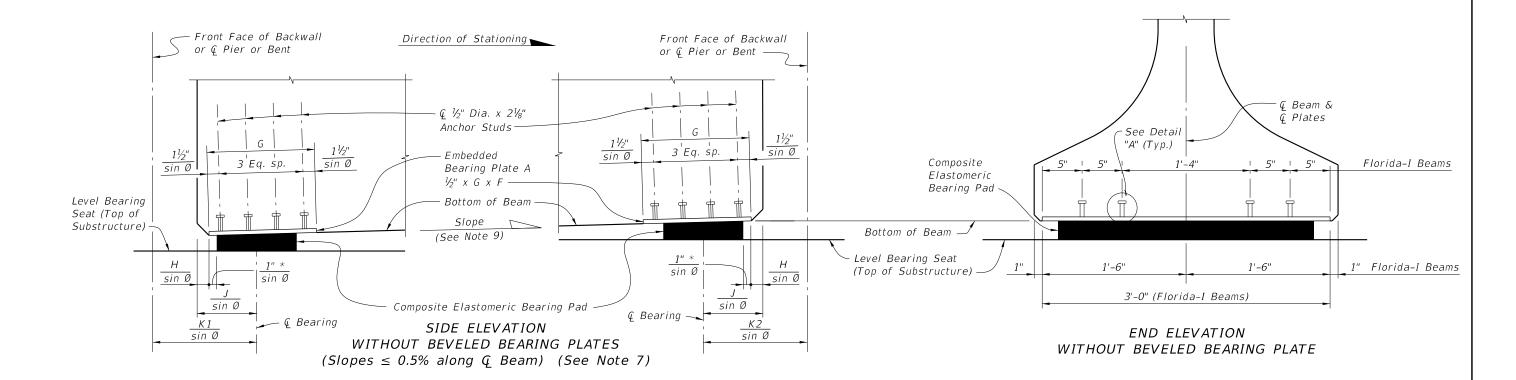
FLORIDA-U BEAMS - BUILD-UP & DEFLECTION DATA INDEX

SHEET

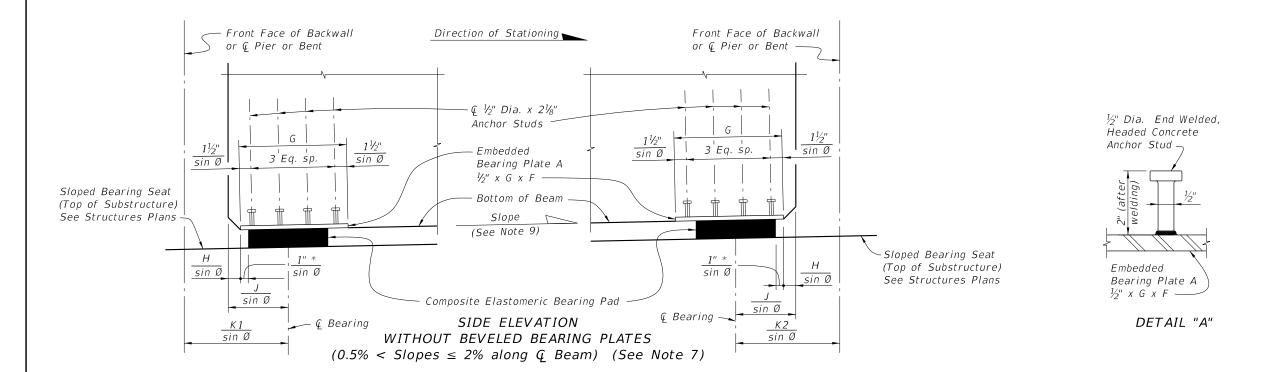
450-299 1 of 1







* ½" Pad Type K



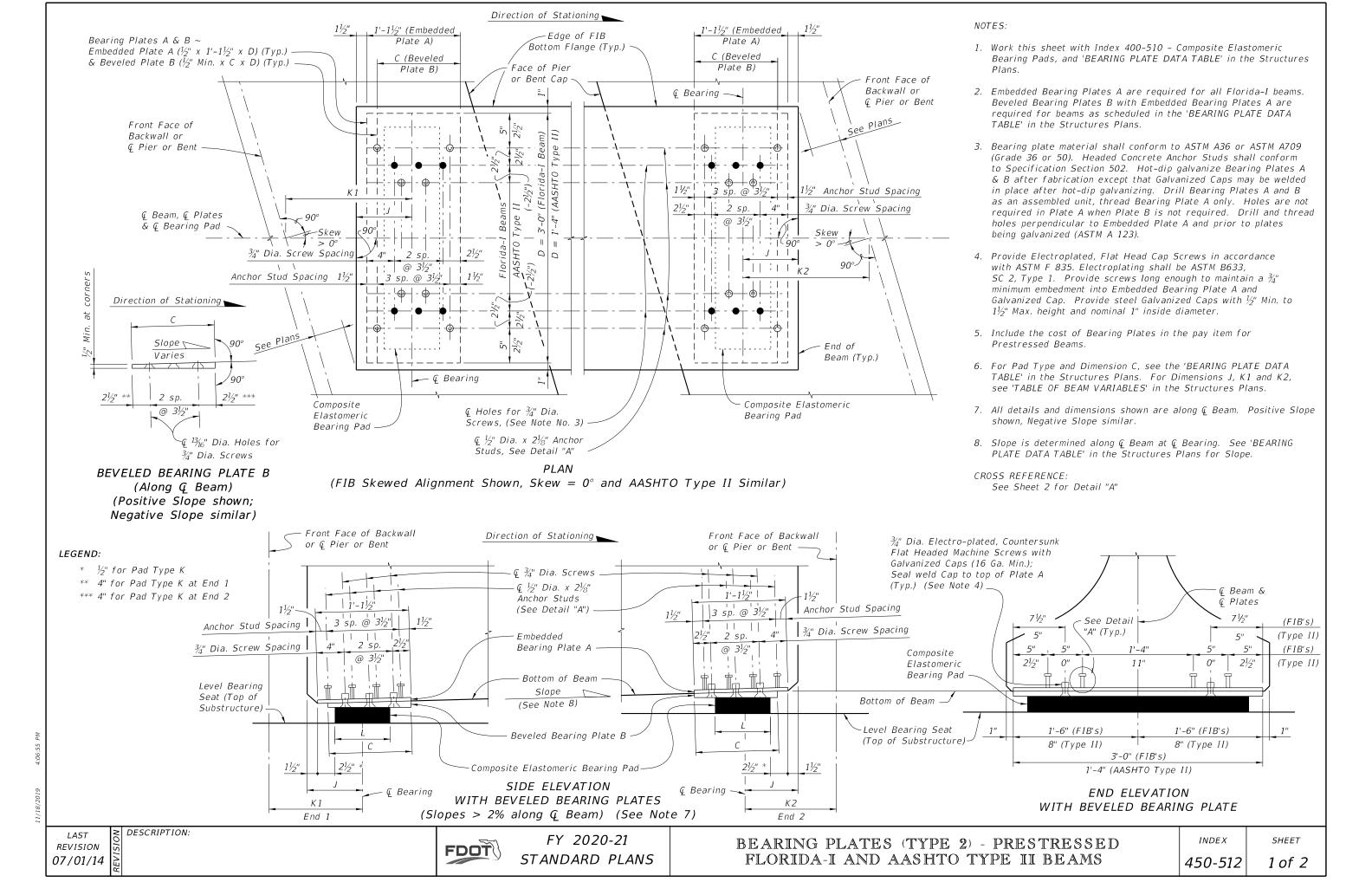
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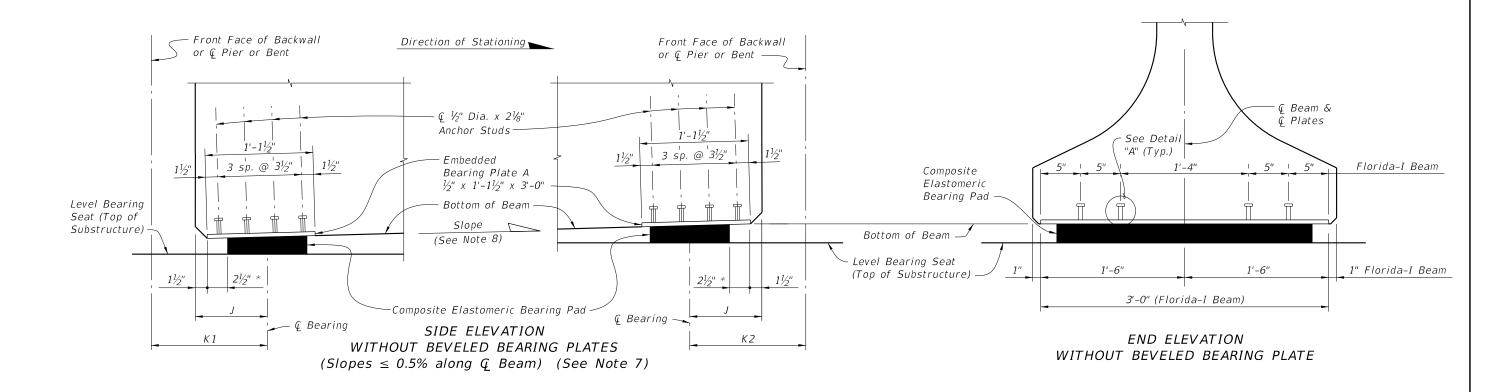
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See Sheet 1 for dimension H and Notes.

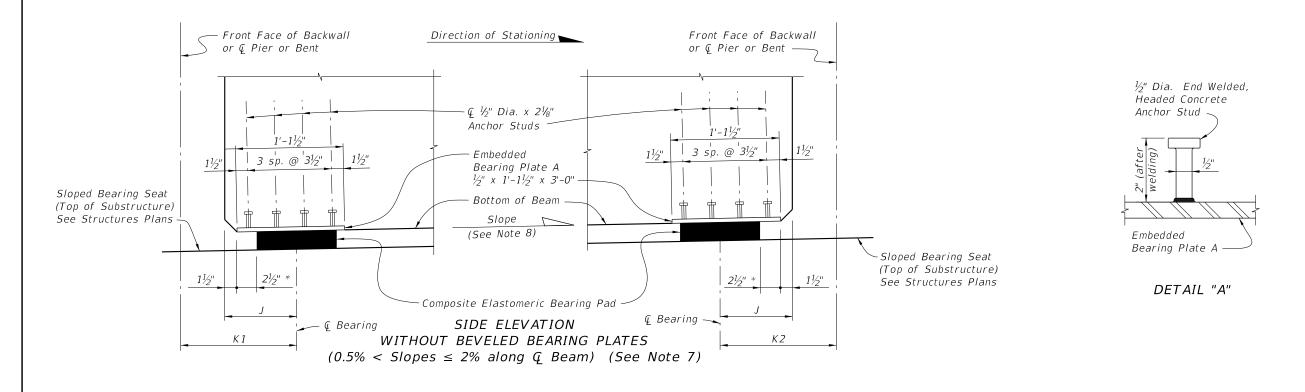
LAST REVISION 07/01/14

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* ½" for Pad Type K



CROSS REFERENCE: See Sheet 1 for Notes.

REVISION 07/01/14

DESCRIPTION:

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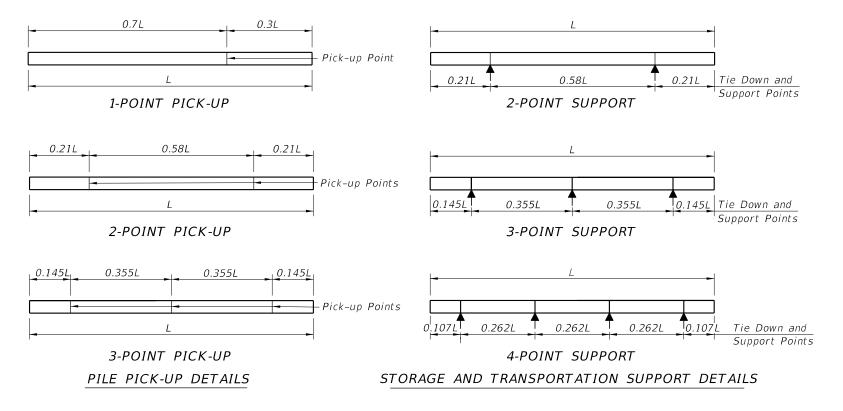
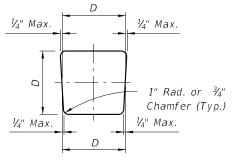
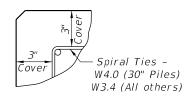


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D =	= Squa	are Pil	e Size	e (inch	nes)	Required Storage and Transportation Detail	Pick-Up Detail			
	12	14	18	20	24	30					
Maximum	48	52	59	62	68	87	2, 3, or 4 point	1 Point			
Pile Length (Feet)	69	75	85	89	98	124	2, 3, or 4 point	2 Point			
	99	107	121	128	140	178	3 or 4 point	3 Point			



TYPICAL PILE SHAPE FOR MOLD FORMS



DETAIL SHOWING TYPICAL COVER

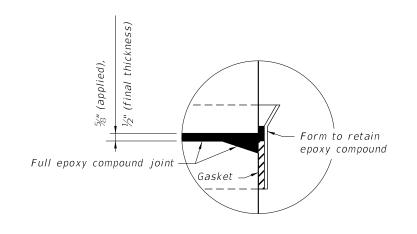
PRESTRESSED CONCRETE PILE NOTES:

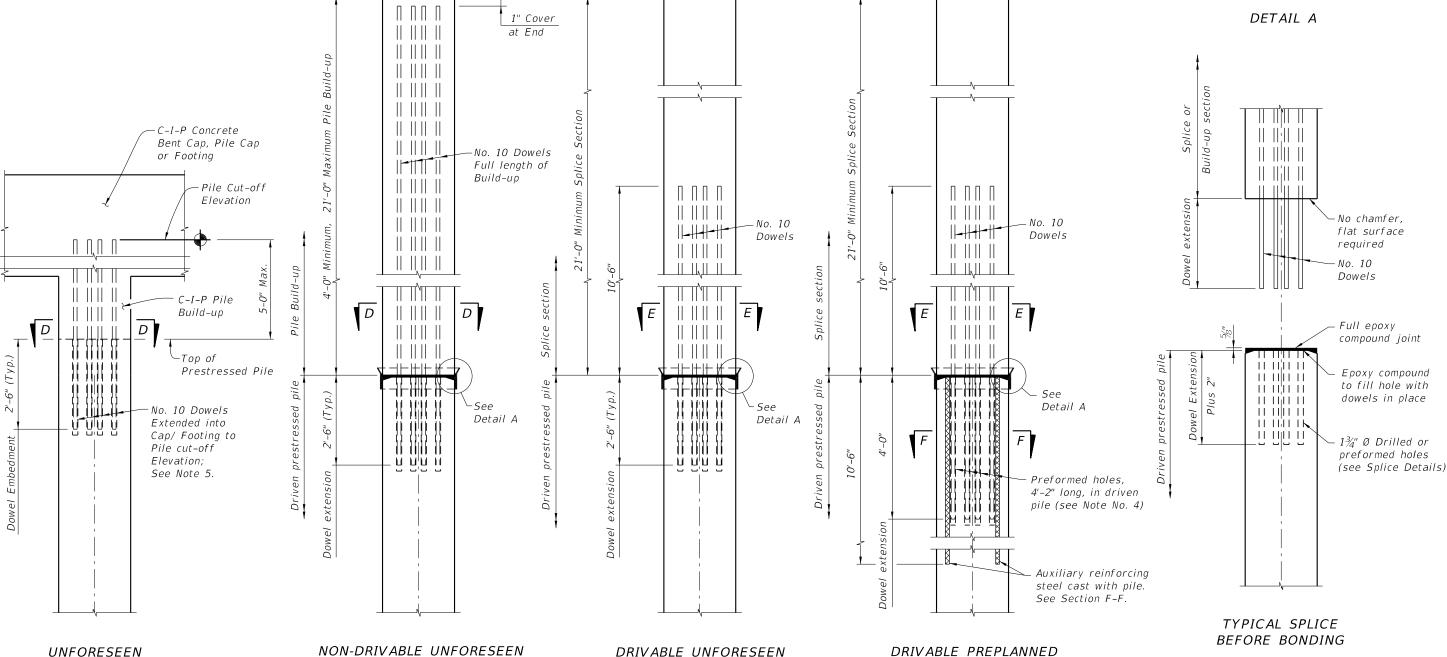
- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V (Special), except use Class VI for High Moment Capacity Pile (Index 455-031).
 - B. High Capacity Splice Collar: Class V (Special).
 - C. Silica Fume: See "GENERAL NOTES" in the Structures Plans for locations where the use of silica fume, metakaolin or ultra-fine flyash is required.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
 - B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
 - A. Bars: Meet the requirements of Specification Section 415.
 - . Prestressing Strands: Meet the requirements of Specification Section 933.
 - Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
- B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 962. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

A-06-57 PM

- 1. For Sections D-D, E-E, & F-F see Index 455-012 thru 455-030 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- 3. In cases where pile splices are desired due to length limitations in shipping and/or handling, the "Drivable Preplanned Prestressed Precast Splice Detail" shall be used.

 Mechanical Pile Splices contained on the Approved Products List (APL) may also be used.
- 4. When preformed dowel holes are utilized, the 1" spiral tie pitch shall be continued to 4'-0" below the head of the pile, See Index 455-018, 455-020 & 455-024. Preformed holes shall utilize either removable preforming material or stay-in-place corrugated galvanized steel ducts. Stay-in-place ducts shall be fabricated from galvanized sheet steel meeting the requirements of ASTM A653, Coating Designation G90, 26 gauge. Ducts shall be 2" diameter with a minimum corrugation (rib) height of 0.12 in. Ducts shall be fabricated with either welded or interlocked seams. Galvanizing of welded seams will not be required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 10 Dowels into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.





PRESTRESSED PRECAST

PILE SPLICE DETAIL

REVISION

07/01/14

REINFORCED C-I-P

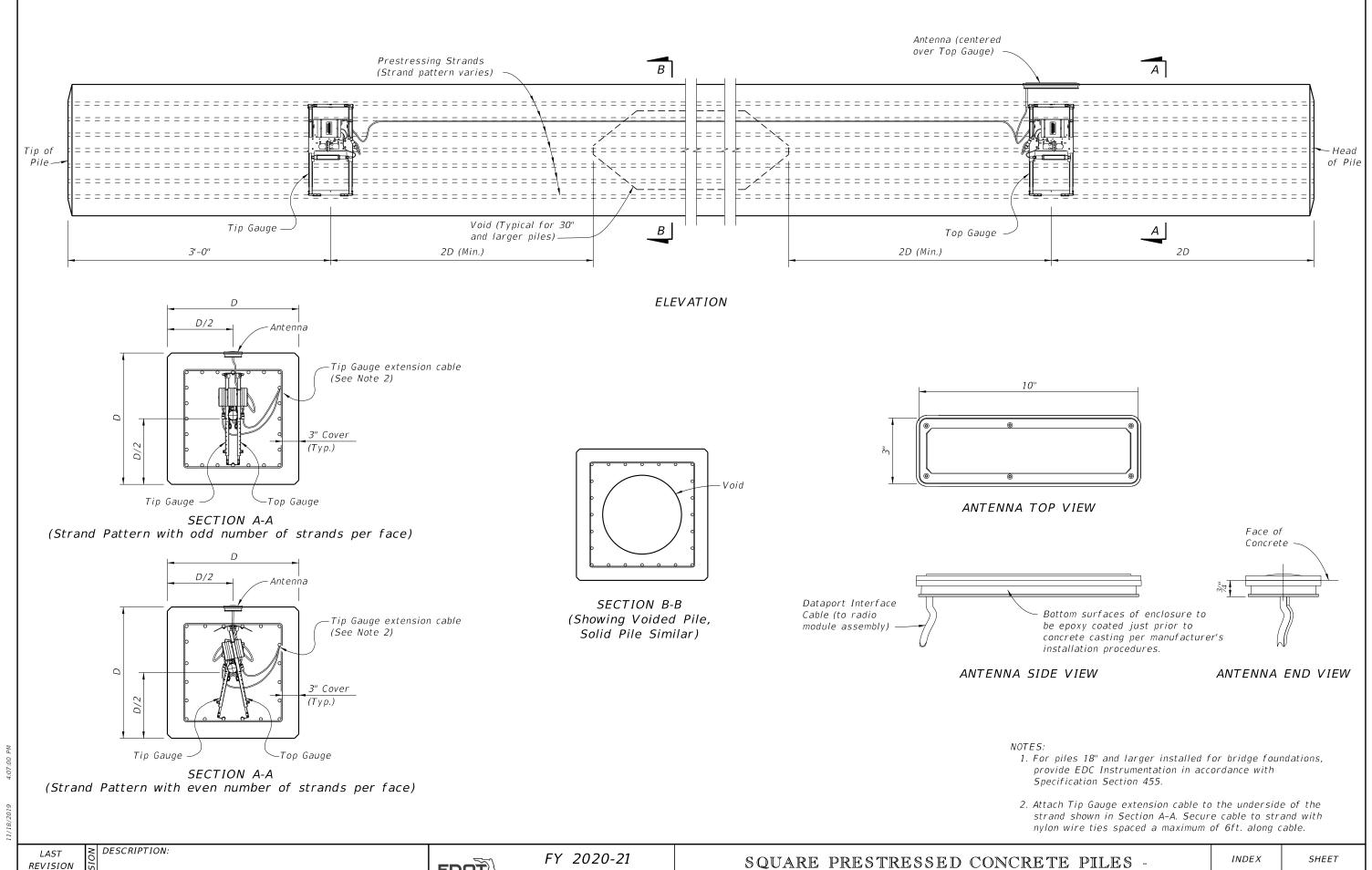
PILE BUILD-UP DETAIL

REINFORCED PRECAST

PILE BUILD-UP DETAIL

PRESTRESSED PRECAST

PILE SPLICE DETAIL



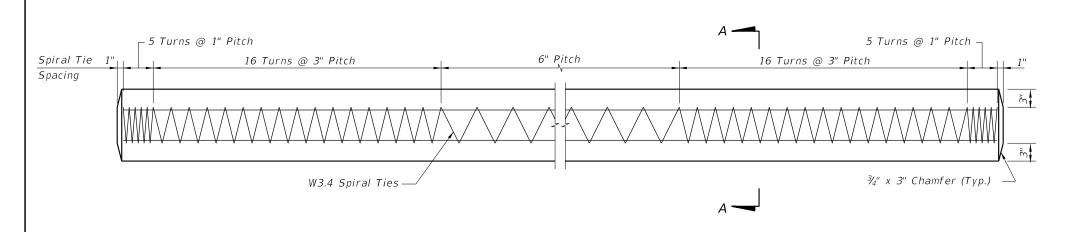
07/01/15

STANDARD PLANS

SQUARE PRESTRESSED CONCRETE PILES -EDC INSTRUMENTATION

455-003

1 of 1



ALTERNATE STRAND PATTERNS

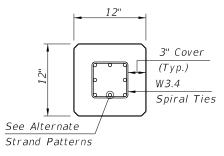
4 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

 $8 \sim \frac{1}{2}$ 0 (Special), Grade 270 LRS, at 25 kips

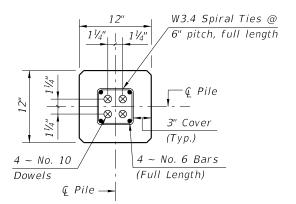
 $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 24 kips

 $8 \sim \overline{/\!\!/}_{16}$ " Ø, Grade 270 LRS, at 23 kips

12 ~ ¾" Ø, Grade 270 LRS, at 16 kips

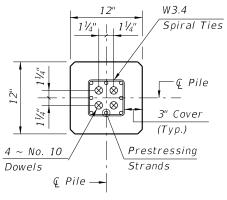


SECTION A-A



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

NOTES:

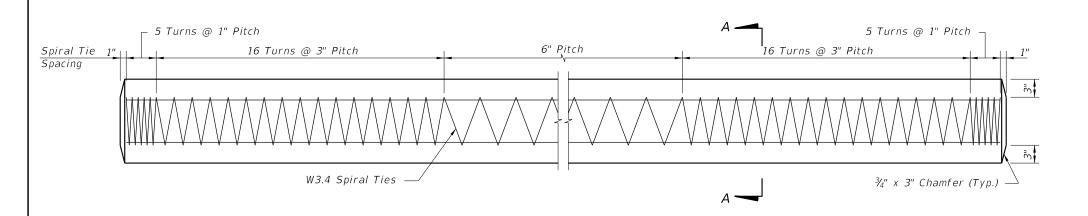
- 1. Work this Index with Index 450-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

 Place one strand at each corner and place the remaining

strands equally spaced between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.

DESCRIPTION:

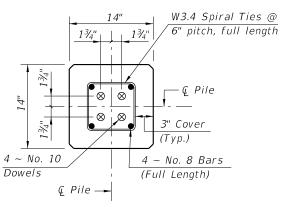


3" Cover (Typ.) W3.4 Spiral Ties See Alternate Strand Patterns

SECTION A-A

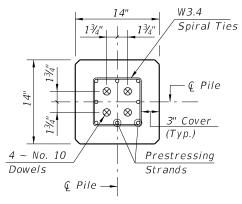
ALTERNATE STRAND PATTERNS

- 8 ~ 0.6" Ø, Grade 270 LRS, at 33 kips
- $8 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 31 kips
- $8 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips
- $12 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips
- 16 ~ ¾" Ø, Grade 270 LRS, at 16 kips



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

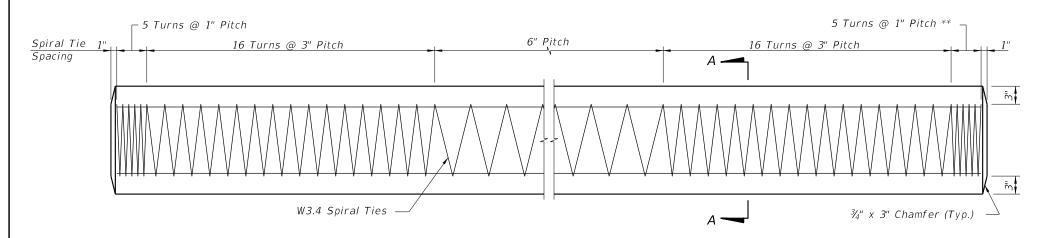
PILE SPLICE REINFORCEMENT DETAILS

NOTES:

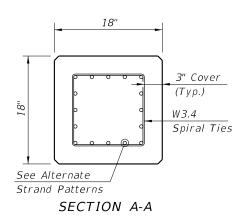
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

DESCRIPTION:



** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips

 $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

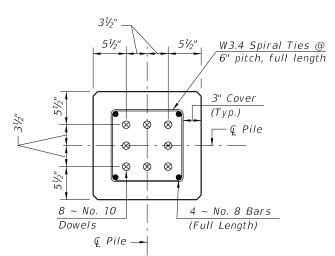
 $16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips

 $20 \sim \frac{7}{16}$ Ø, Grade 270 LRS, at 21 kips

 $24 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 17 kips

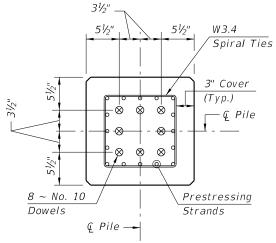
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



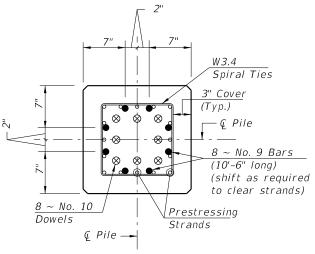
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

REVISION 01/01/12

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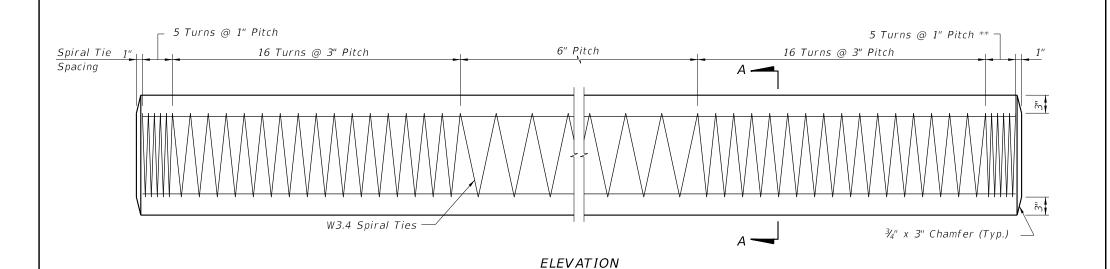
FDOT

FY 2020-21 STANDARD PLANS

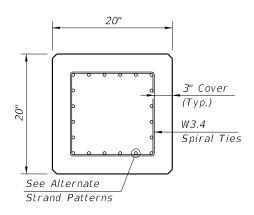
INDEX

SHEET

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** See Note 4 on Index 455-002



SECTION A-A

ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, Grade 270 LRS, at 42 kips

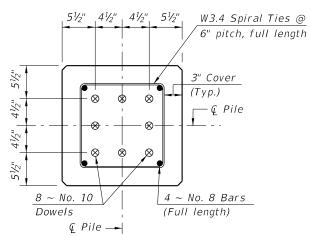
 $16 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 31 kips

 $16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips

 $24 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips

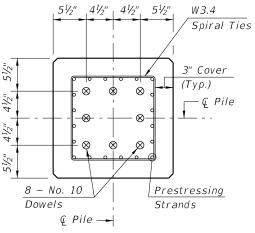
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



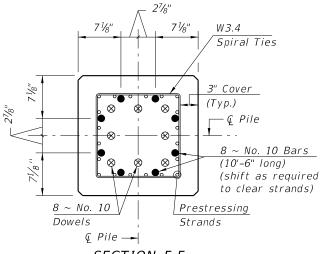
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

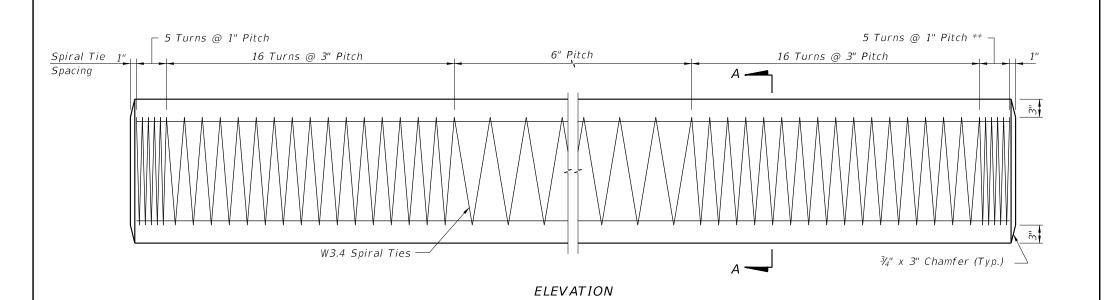
PILE SPLICE REINFORCEMENT DETAILS

REVISION 01/01/12

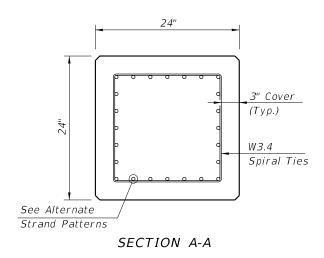
DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS



** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

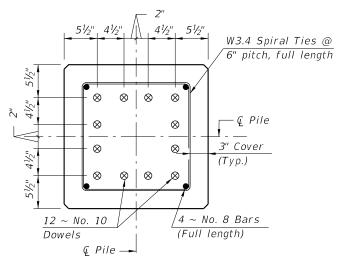
16 ~ 0.6" Ø, Grade 270 LRS, at 44 kips

 $20 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

 $24 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 31 kips

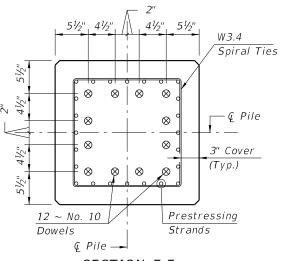
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



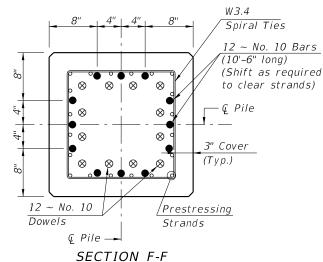
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



(See Drivable Preplanned Pile Splice Detail)

REVISION 01/01/12

DESCRIPTION:

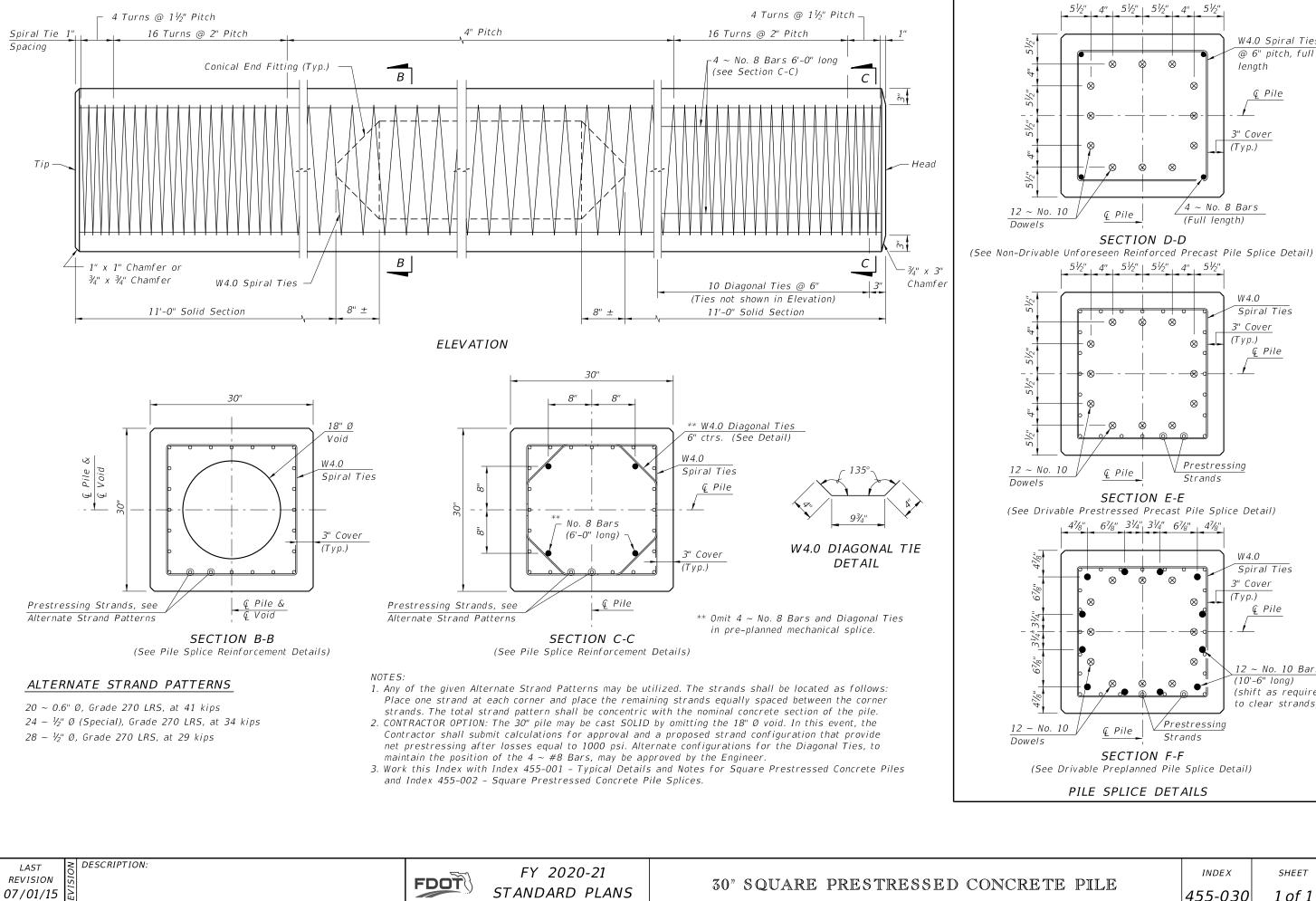
FDOT

FY 2020-21 STANDARD PLANS

24" SQUARE PRESTRESSED CONCRETE PILE

INDEX

SHEET



W4.0 Spiral Ties

@ 6" pitch, full

€ Pile

length

3" Cover

W4.0

3" Cover (Typ.)

W4.0

Spiral Ties 3" Cover (Typ.)

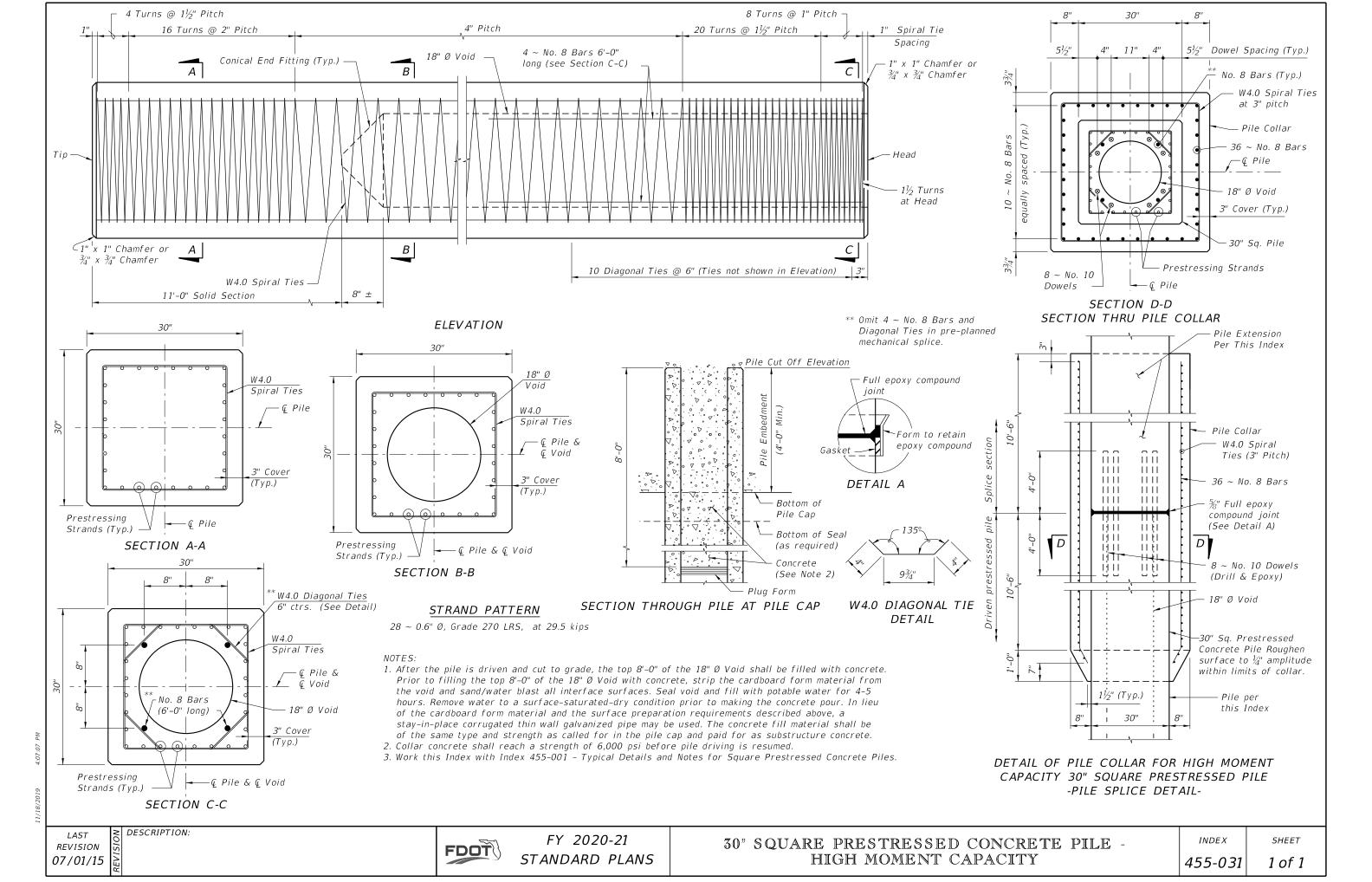
12 ~ No. 10 Bars (10'-6" long)

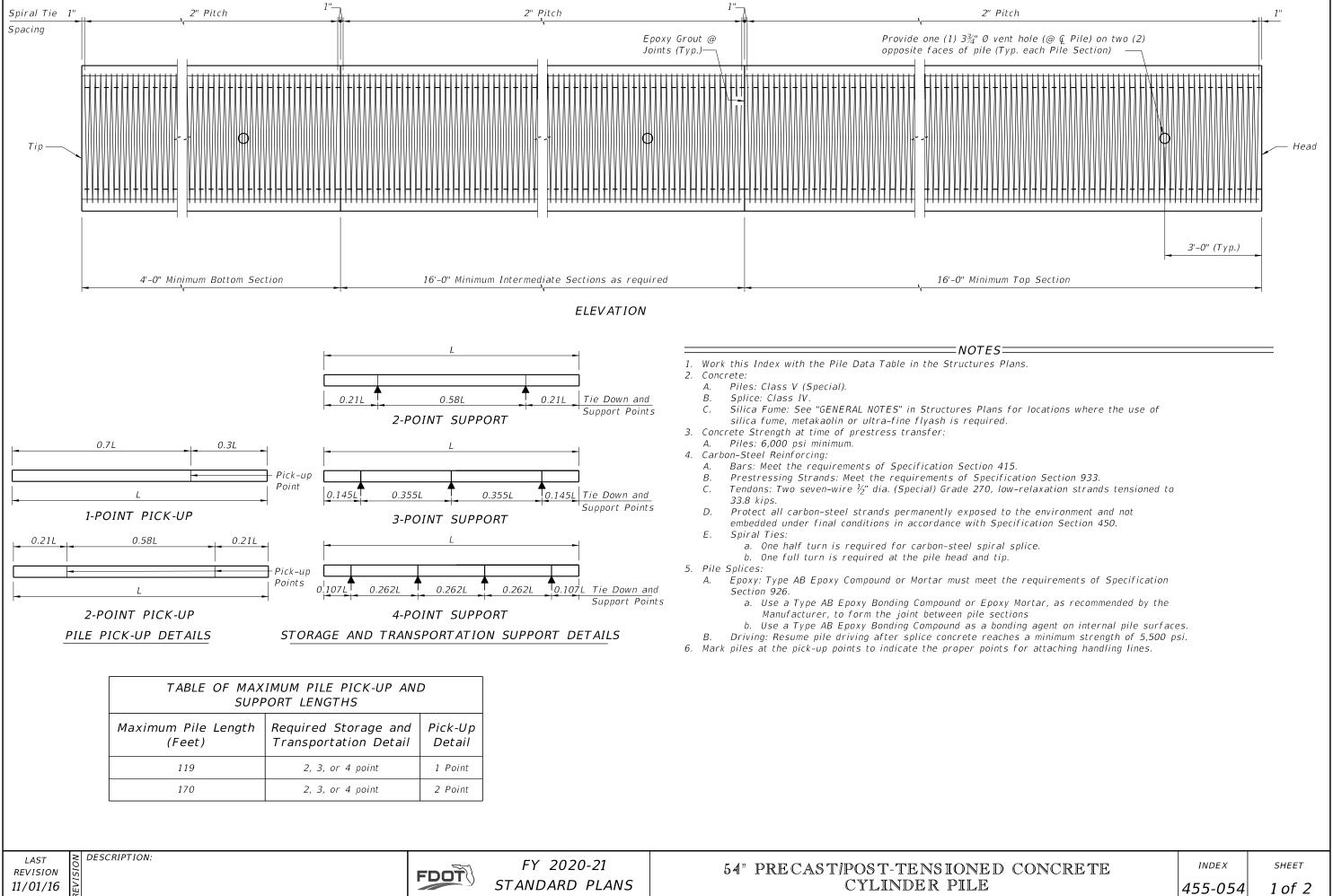
(shift as required

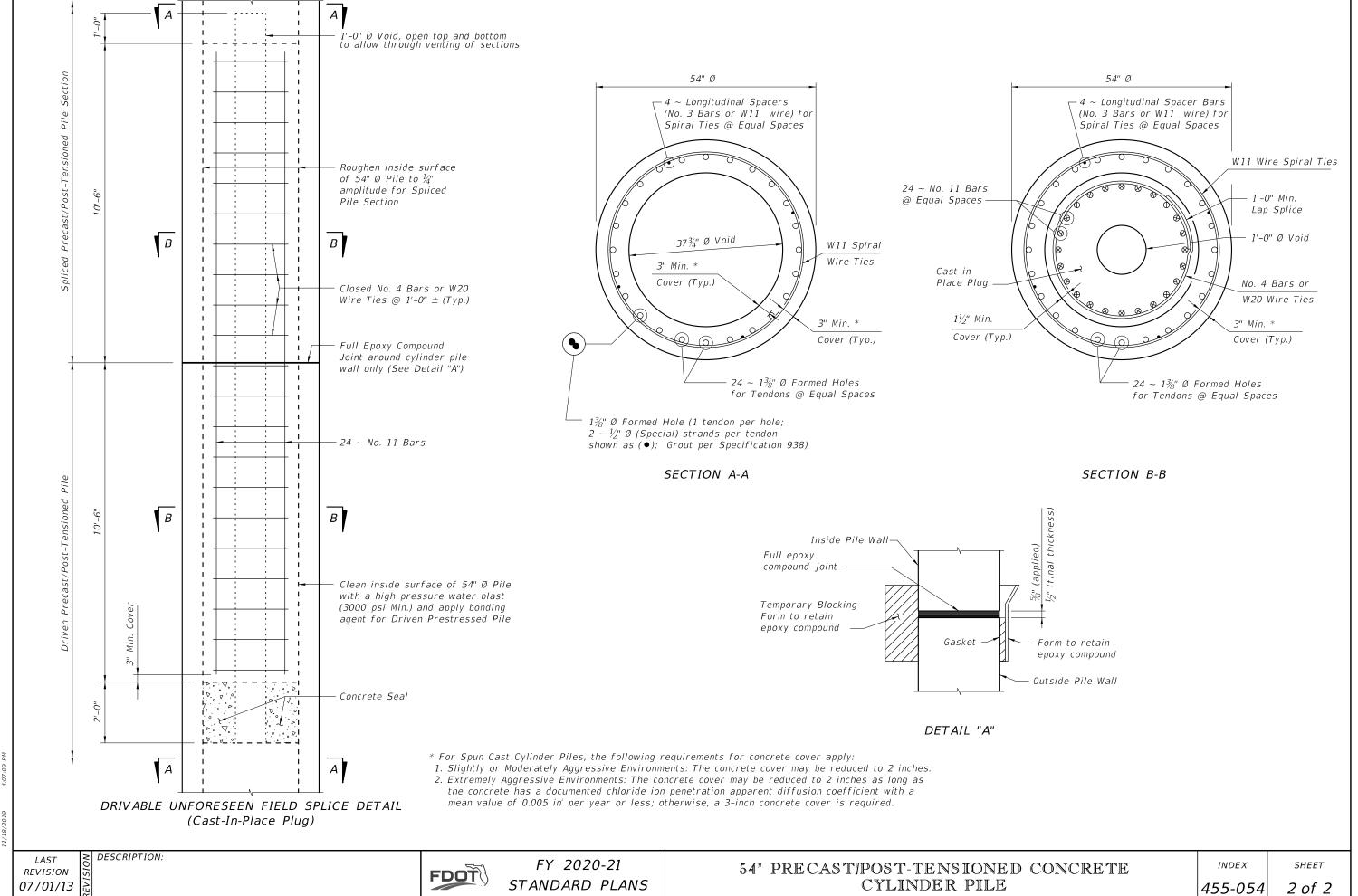
to clear strands)

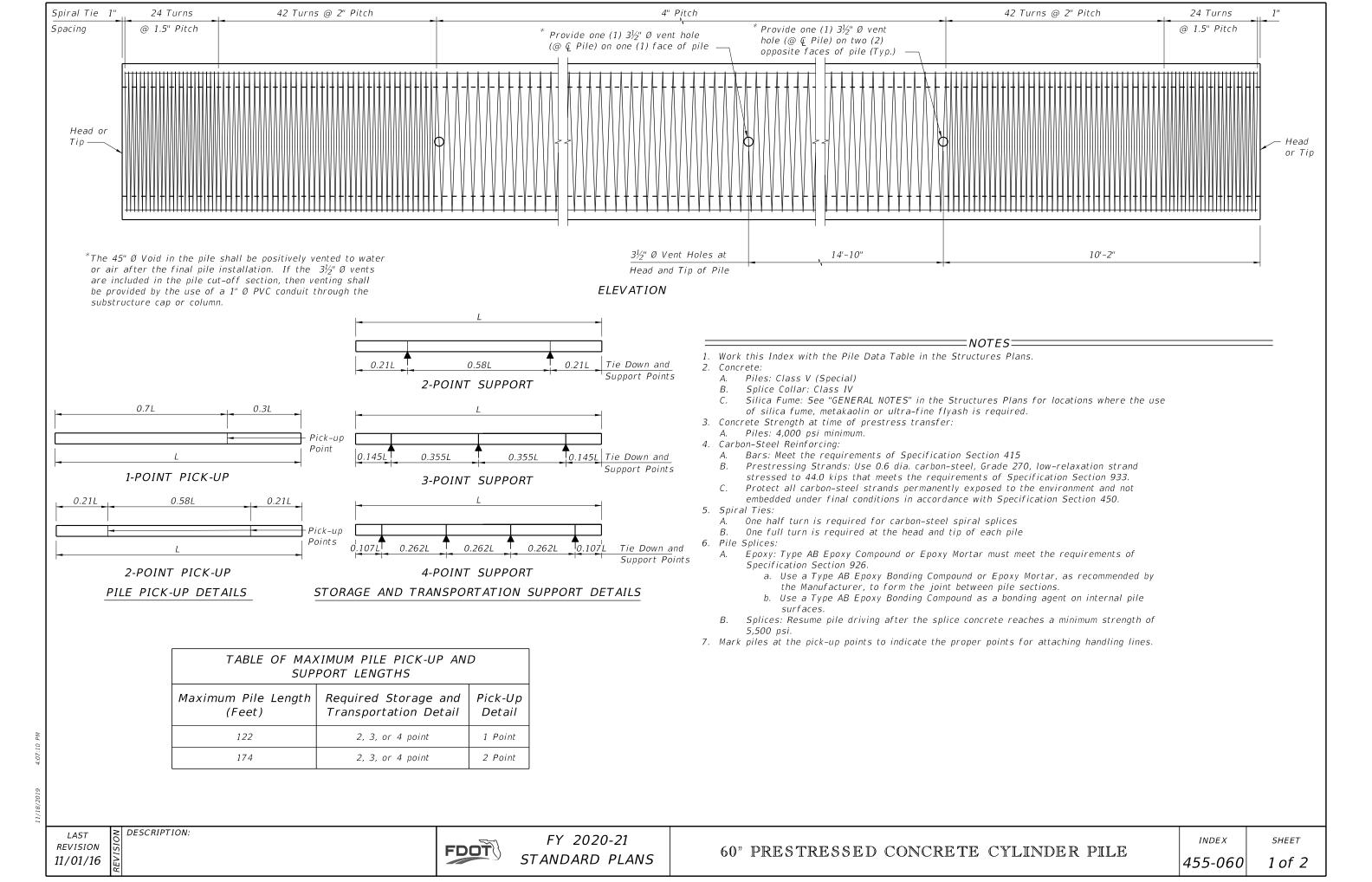
Spiral Ties

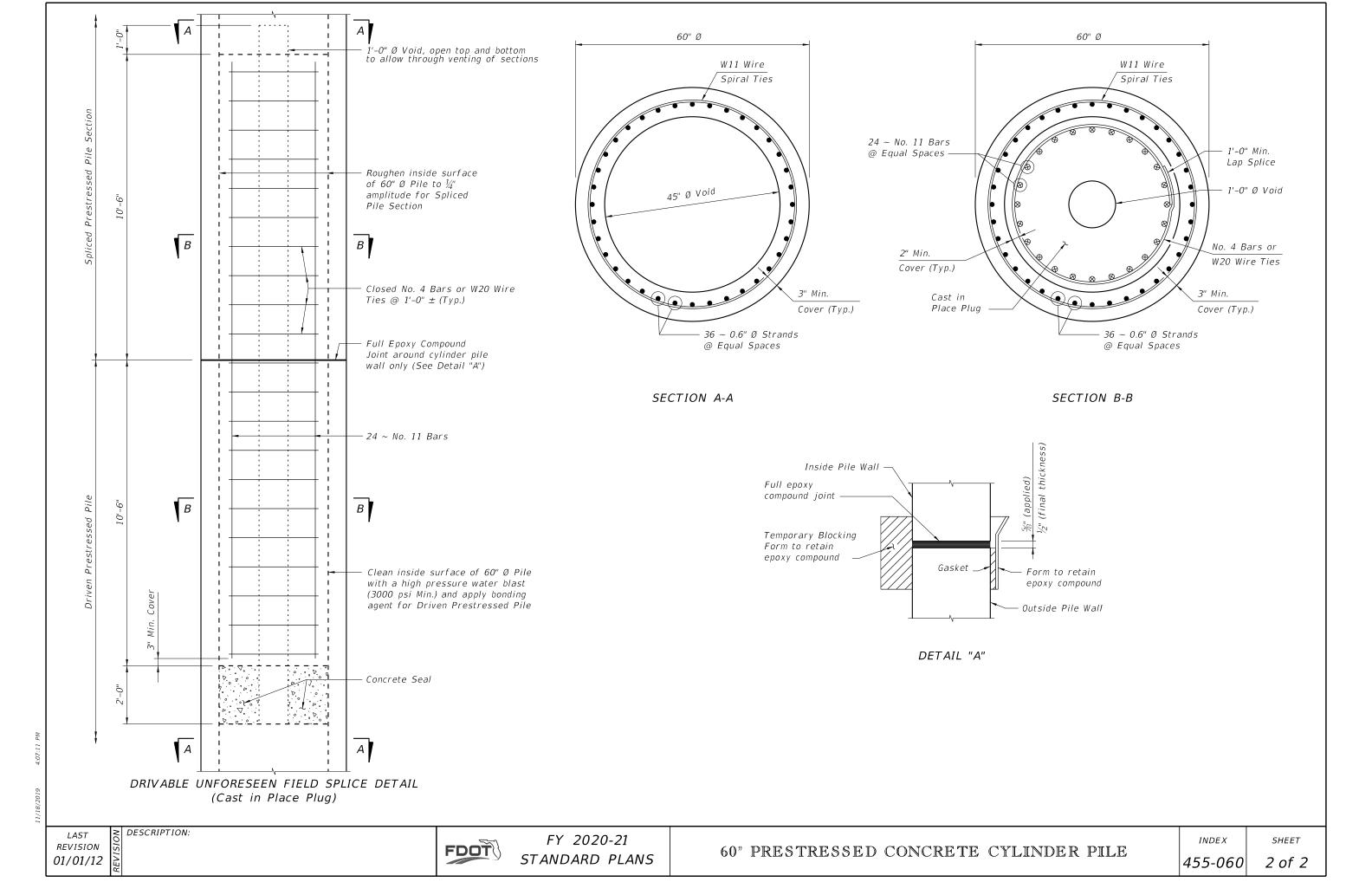
1 of 1











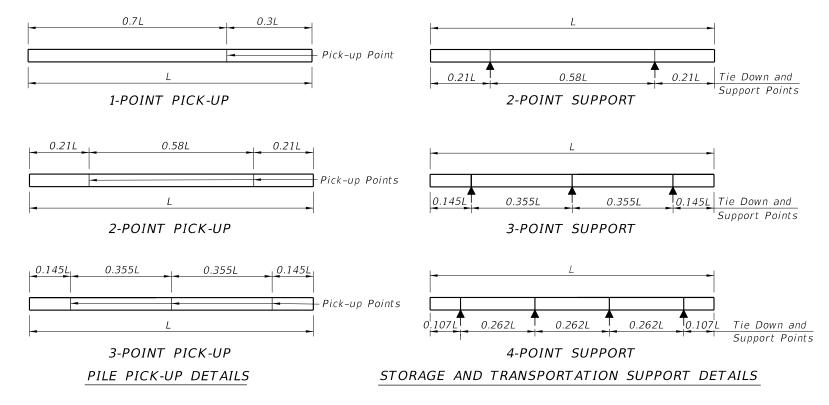
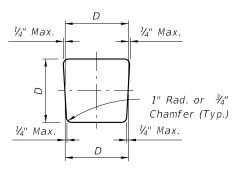
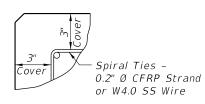


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS											
	D = S	Square	Pile S	ize (in	ches)	Required Storage and Transportation Detail	Pick-Up Detail				
	12	14	18	24	30						
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point				
	69	75	85	98	124	2, 3, or 4 point	2 Point				
	99	107	121	140	178	3 or 4 point	3 Point				



TYPICAL PILE SHAPE FOR MOLD FORMS

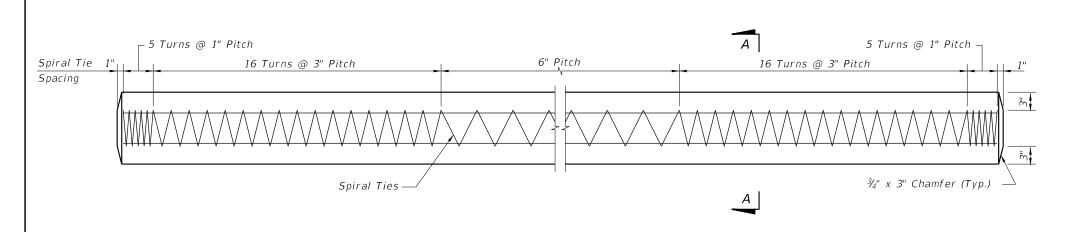


DETAIL SHOWING TYPICAL COVER

PRESTRESSED CONCRETE PILE NOTES:

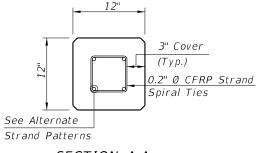
- Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-102), the Prestressed Concrete Pile Standards (Index 455-112, 455-114, 455-118, 455-124, 455-130, and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V (Special)
 - Silica Fume: See "GENERAL NOTES" in the Structures Plans for locations where the use of silica fume, metakaolin or ultra-fine flyash is required for options using stainless steel strand and reinforcing.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
- 4. Reinforcing:
 - A. Bars:
 - a. Stainless Steel: Meet the requirements of Specification Section 931 for Type 304, Grade 75.
 - b. Carbon FRP: Meet the requirements of Specification Section 932.
 - B. Prestressing Strands:
 - a. Stainless Steel: Seven-wire HSSS, UNS S32205 (Type 2205) or UNS S31803 strand, meeting the requirements of Specification Section 933.
 - b. Carbon FRP: Meet the requirements of Specification Section 933.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
 - B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

01/01/16

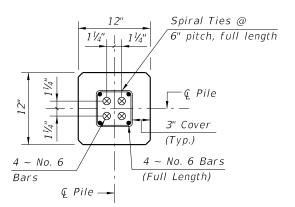


ALTERNATE STRAND PATTERNS

4 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips $4 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 41 kips

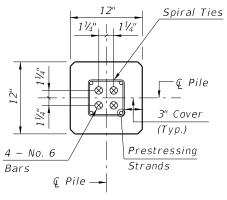


SECTION A-A



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized.

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

DESCRIPTION:

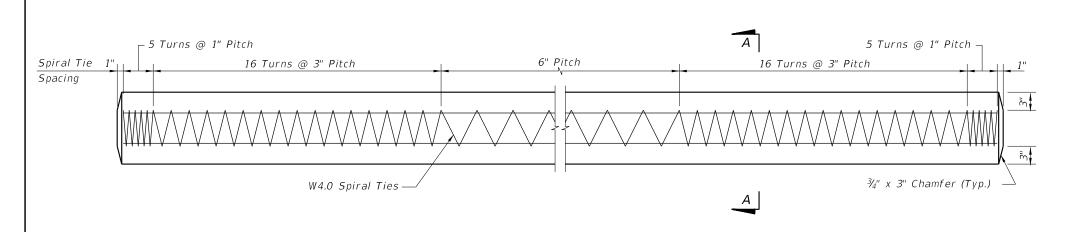
FDOT

FY 2020-21 STANDARD PLANS

12" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX 455-112

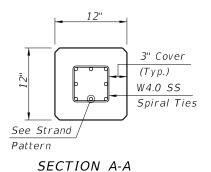
SHEET 1 of 2

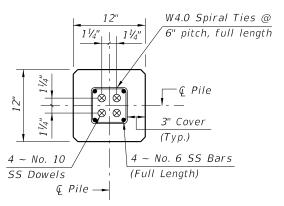


ELEVATION

STRAND PATTERN

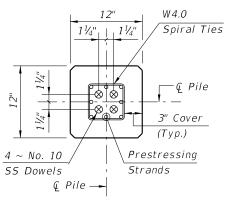
 $8 \sim \frac{1}{2}$ " Ø, HSSS at 24 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

REVISION 01/01/16

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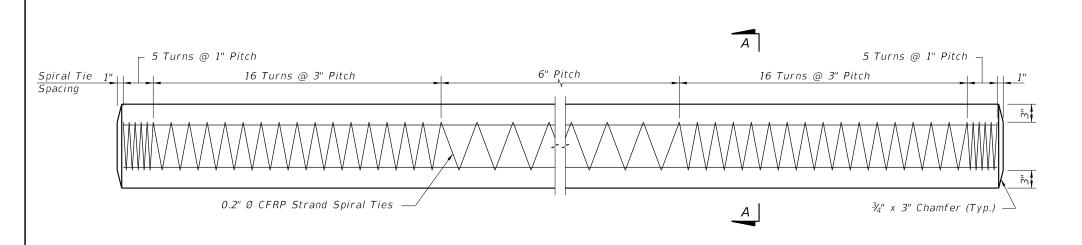
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FY 2020-21 STANDARD PLANS

12" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

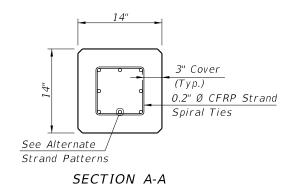
INDEX 455-112

SHEET 2 of 2



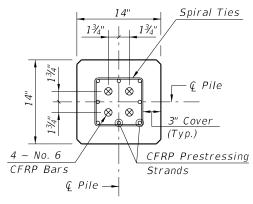
ALTERNATE STRAND PATTERNS

 $8 \sim 0.6$ " Ø, CFRP 7-Strand, at 31.5 kips $8 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 30.5 kips



SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and equally space the remaining strands between the corner strands.

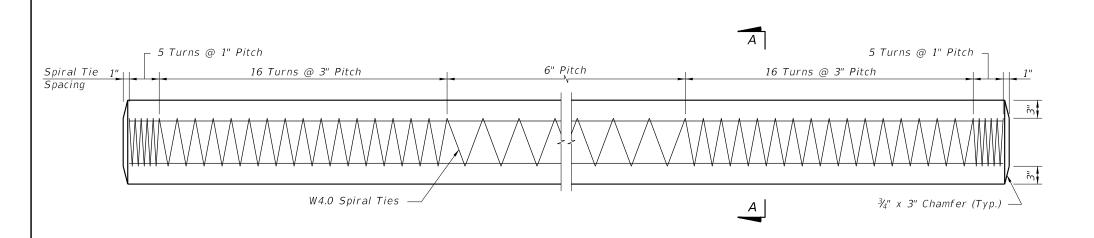
The total strand pattern shall be concentric with the nominal concrete section of the pile.

CFRP PRESTRESSED PILE DETAILS

LAST REVISION 11/01/16

DESCRIPTION:

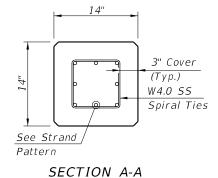
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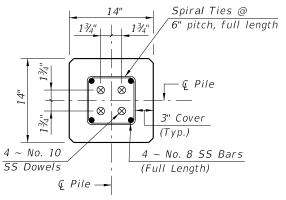


ELEVATION

STRAND PATTERN

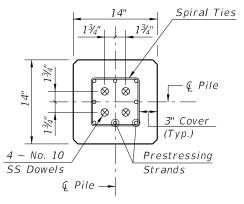
 $12 \sim \frac{1}{2}$ " Ø, HSSS at 23 kips





SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Unforeseen Prestressed Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

SS PRESTRESSED PILE DETAILS

REVISION 01/01/16

DESCRIPTION:

FDOT

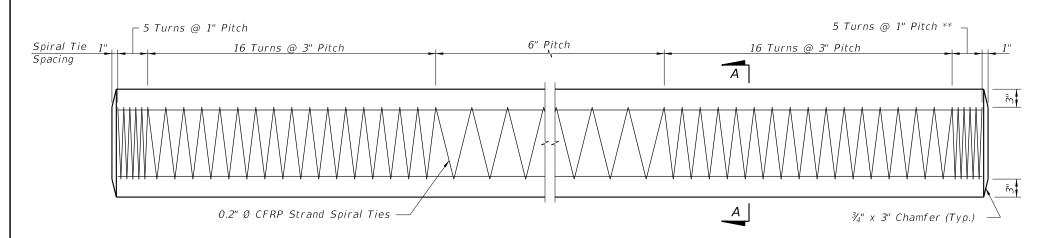
FY 2020-21 STANDARD PLANS

14" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX 455-114

2 of 2

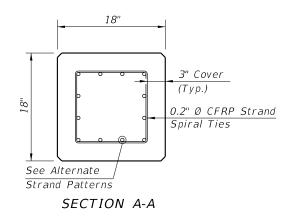
SHEET



** See Note 4 on Index 455-102

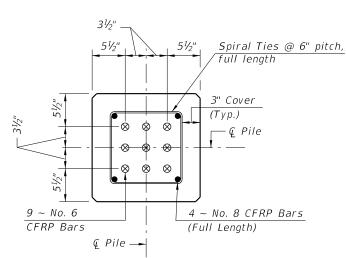
ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, CFRP 7-Strand, at 34 kips $12 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 33 kips



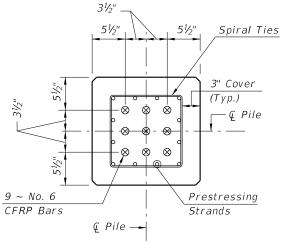
NOTES:

- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 -Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



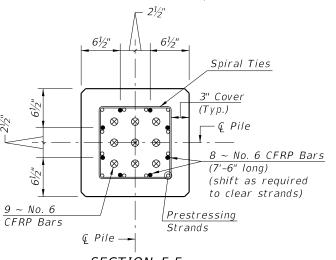
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Splice Detail)

CFRP PILE SPLICE REINFORCEMENT DETAILS

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

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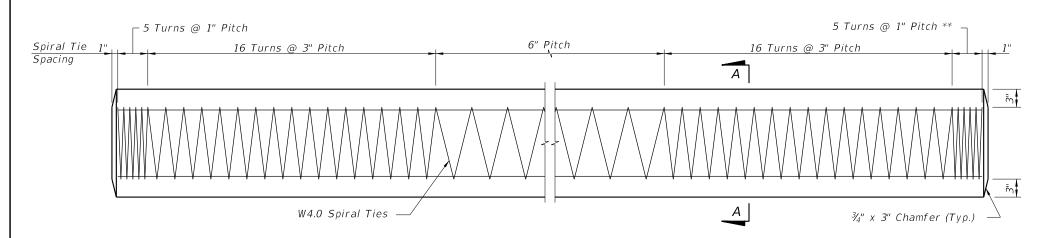
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FY 2020-21 STANDARD PLANS

CONCRETE PILE

INDEX 455-118

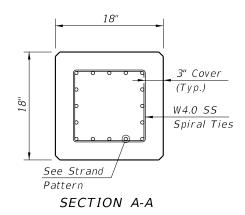
SHEET 1 of 2



** See Note 4 on Index 455-102

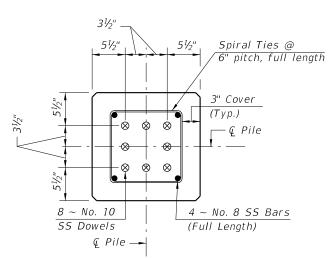
STRAND PATTERN

 $16 \sim \frac{1}{2}$ " Ø, HSSS, at 26 kips



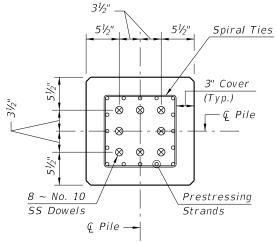
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



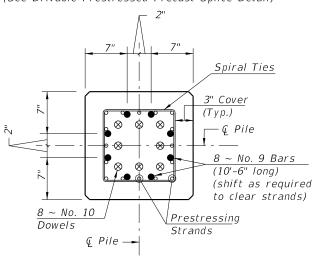
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)



SECTION F-F

(See Drivable Preplanned Predtresses Precast Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 01/01/16

DESCRIPTION:

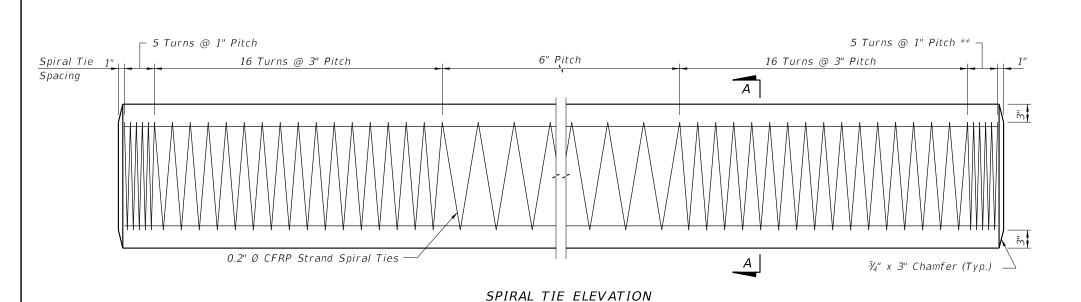
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FY 2020-21 STANDARD PLANS

CONCRETE PILE

INDEX 455-118

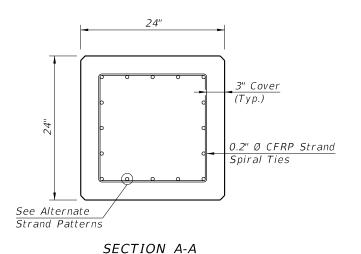
SHEET



** See Note 4 on Index 455-102

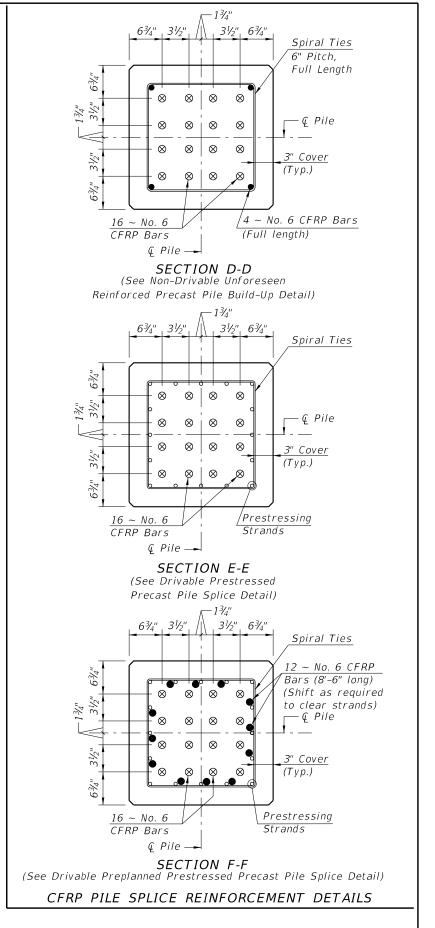
ALTERNATE STRAND PATTERNS

16 ~ 0.6" Ø, CFRP 7-Strand, at 42 kips $16 \sim \frac{1}{2}$ " Ø, CFRP Single-Strand, at 41 kips



- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining

strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

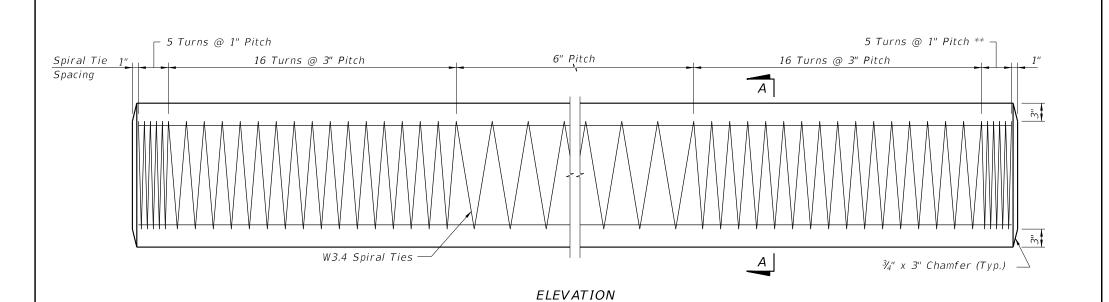
DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

SHEET 1 of 2

455-124



** See Note 4 on Index 455-102

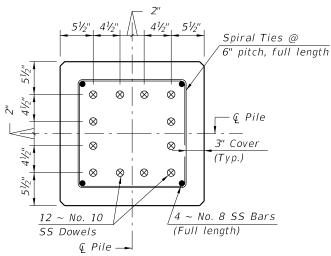
3" Cover (Typ.)W4.0 SS Spiral Ties See Strand Pattern SECTION A-A

STRAND PATTERN

 $28 \sim \frac{1}{2}$ " Ø, HSSS at 26 kips

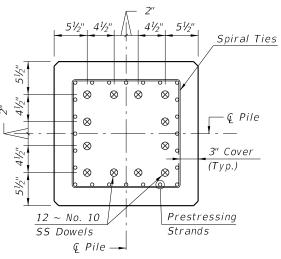
- 1. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.
- 2. Any of the given Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands.

The total strand pattern shall be concentric with the nominal concrete section of the pile.



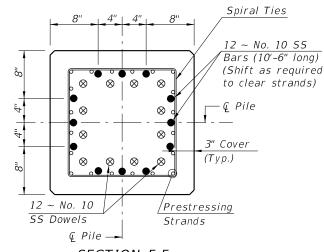
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Pile Splice Detail)

SS PILE SPLICE REINFORCEMENT DETAILS

SS PRESTRESSED PILE DETAILS

REVISION 01/01/16

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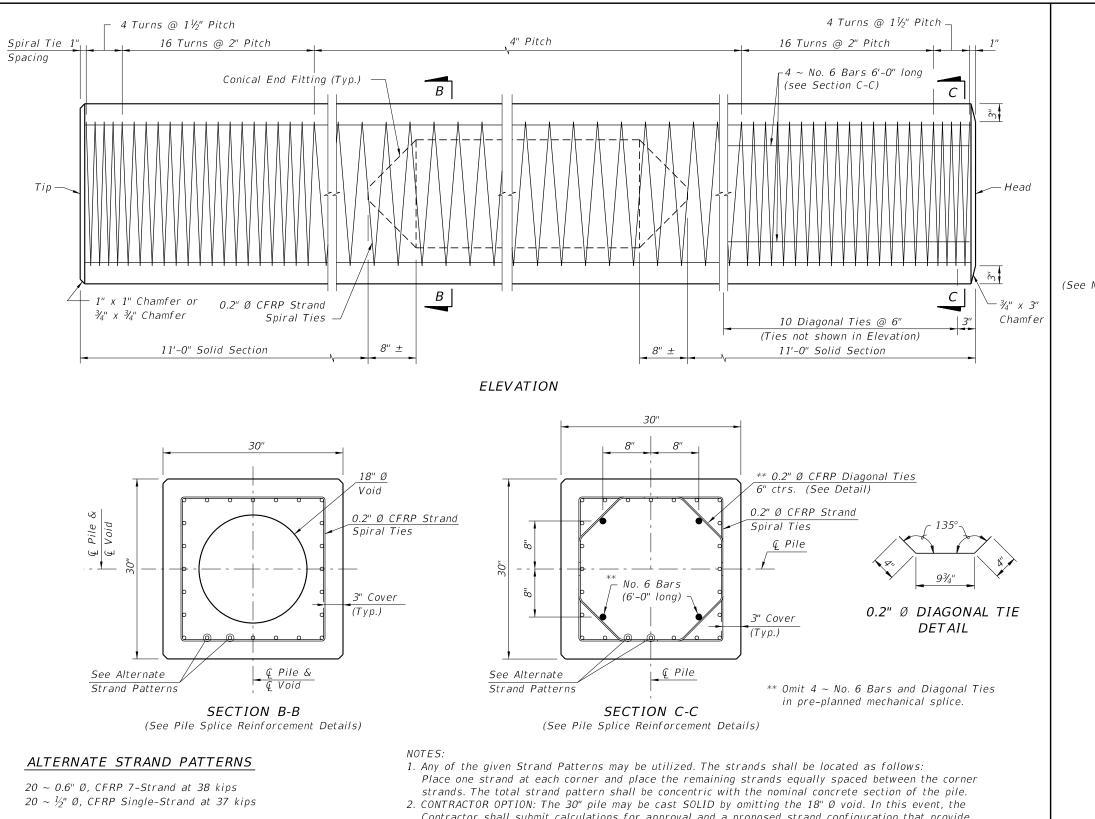
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FY 2020-21 STANDARD PLANS

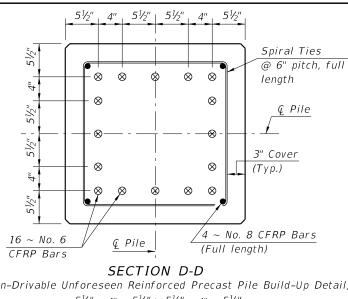
24" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

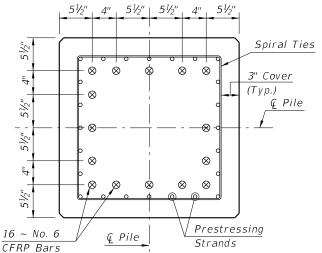
SHEET 2 of 2



- Contractor shall submit calculations for approval and a proposed strand configuration that provide net prestressing after losses equal to 1000 psi. Alternate configurations for the Diagonal Ties, to maintain the position of the $4\sim$ #6 Bars, may be approved by the Engineer.
- 3. Work this Index with Index 455-101 Typical Details and Notes for Square CFRP & SS Prestressed Concrete Piles and Index 455-102 - Square CFRP & SS Prestressed Concrete Pile Splices.

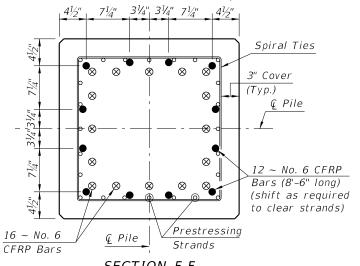


(See Non-Drivable Unforeseen Reinforced Precast Pile Build-Up Detail)



SECTION E-E

(See Drivable Prestressed Precast Pile Splice Detail)



SECTION F-F

(See Drivable Preplanned Prestressed Precast Pile Splice Detail)

CFRP PILE SPLICE DETAILS

CFRP PRESTRESSED PILE DETAILS

REVISION 11/01/16

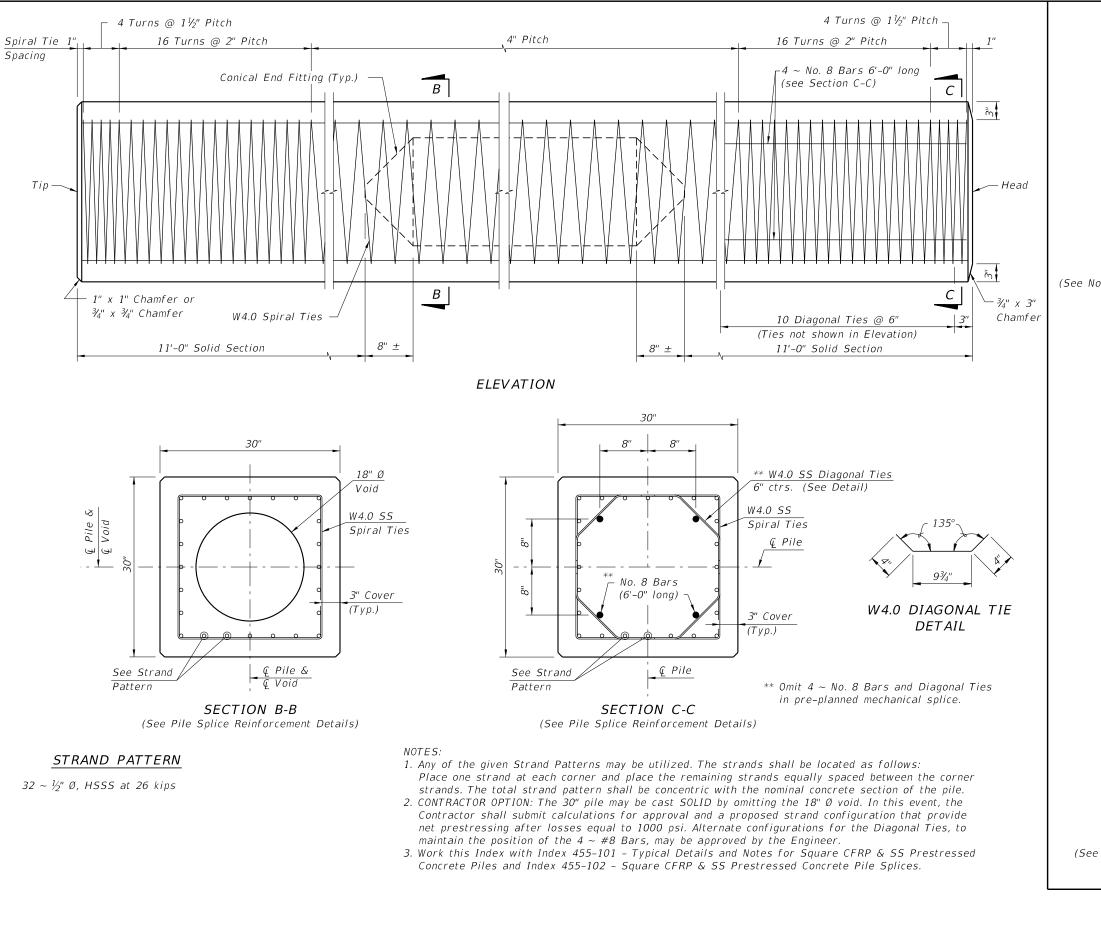
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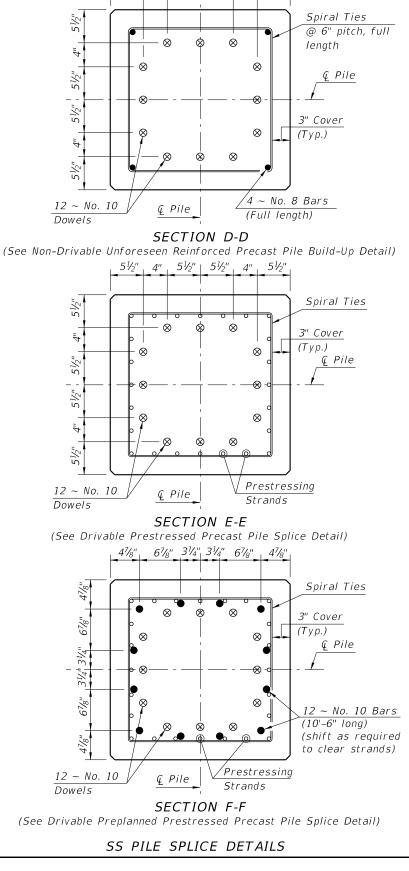
FDOT

FY 2020-21 STANDARD PLANS 30" SQUARE CFRP & SS PRESTRESSED CONCRETE PILE

INDEX

SHEET





 $5\frac{1}{2}$ " | 4" | $5\frac{1}{2}$ " | $5\frac{1}{2}$ " | 4" | $5\frac{1}{2}$ "

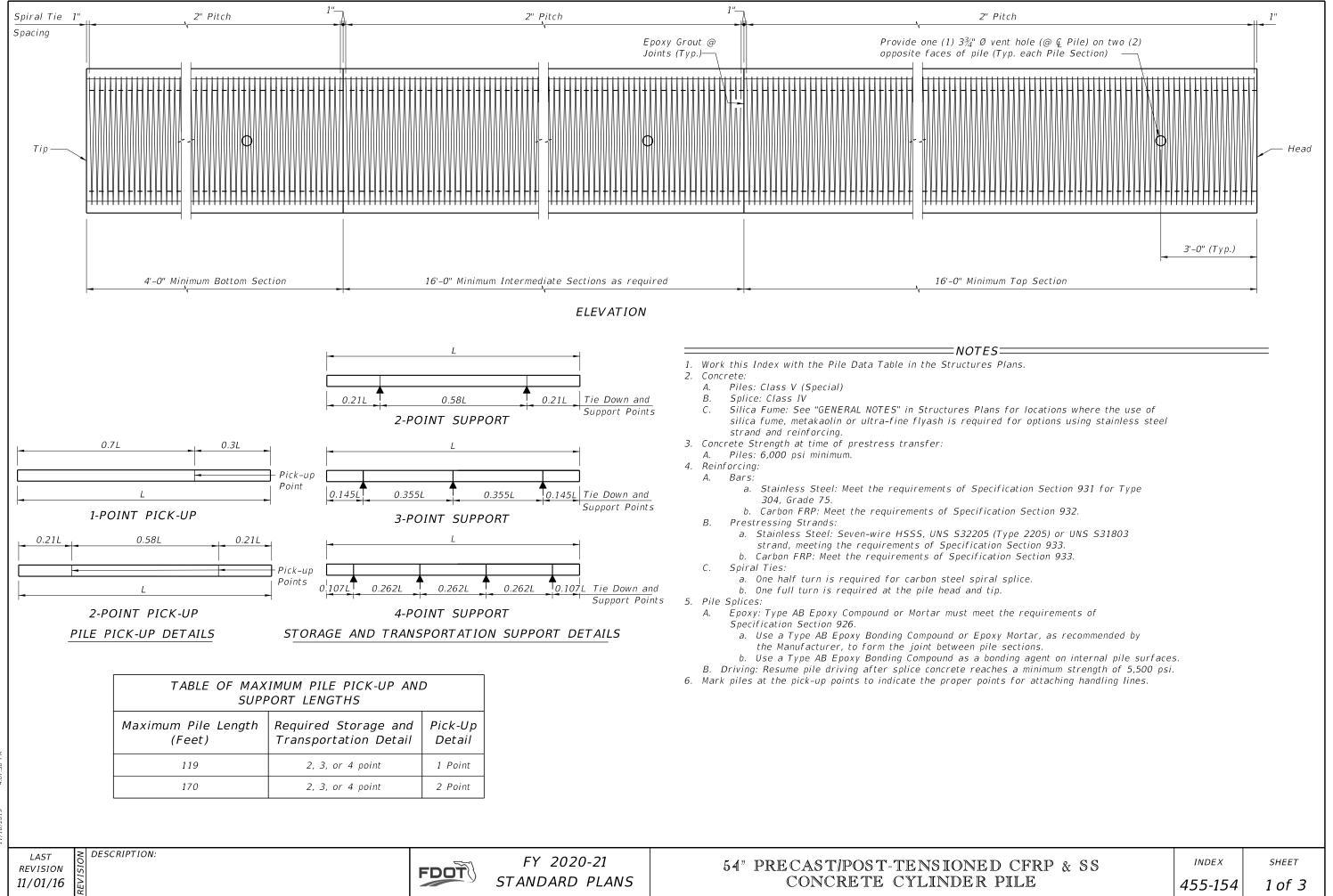
SS PRESTRESSED PILE DETAILS

LAST REVISION 11/01/16

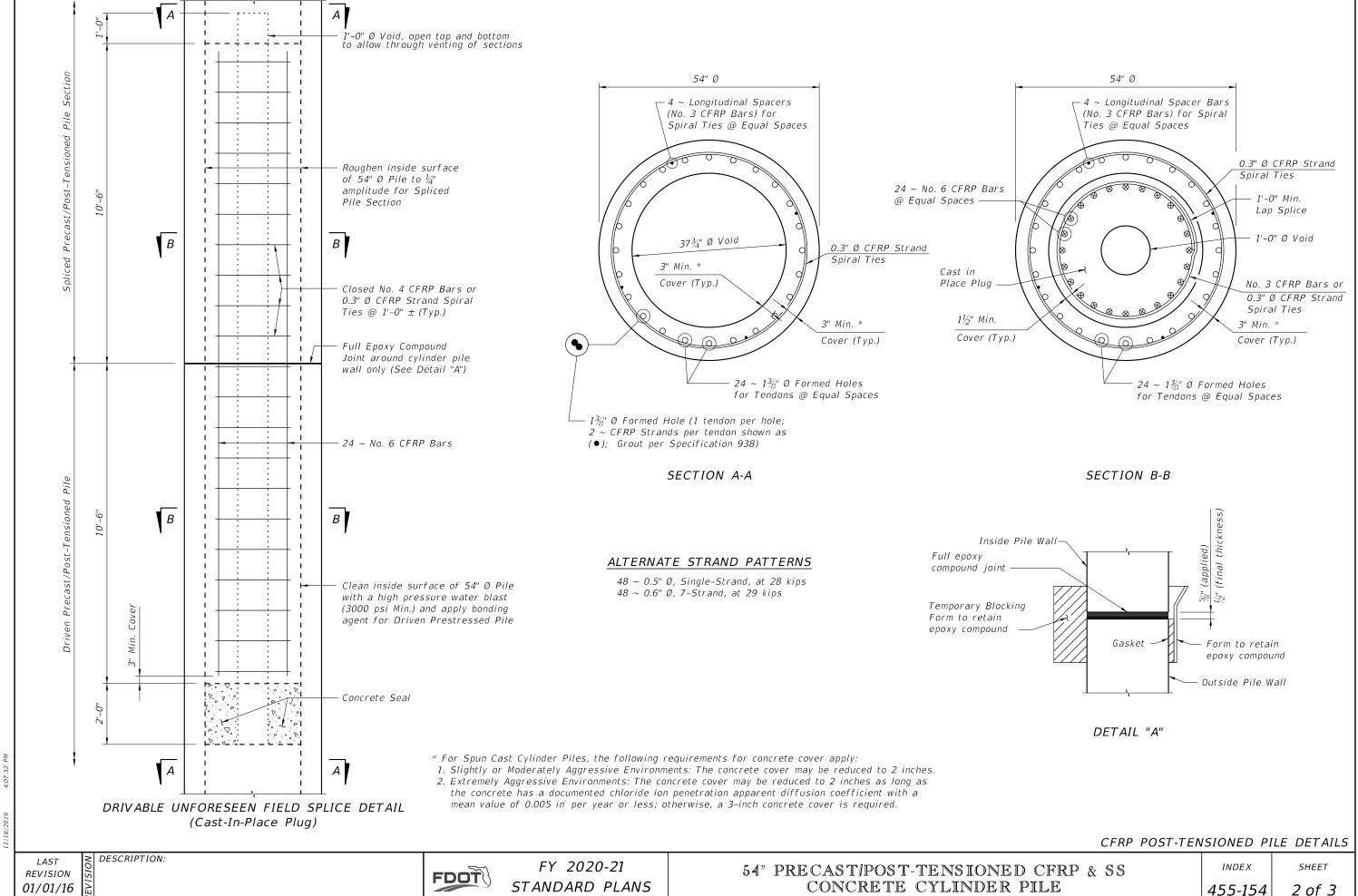
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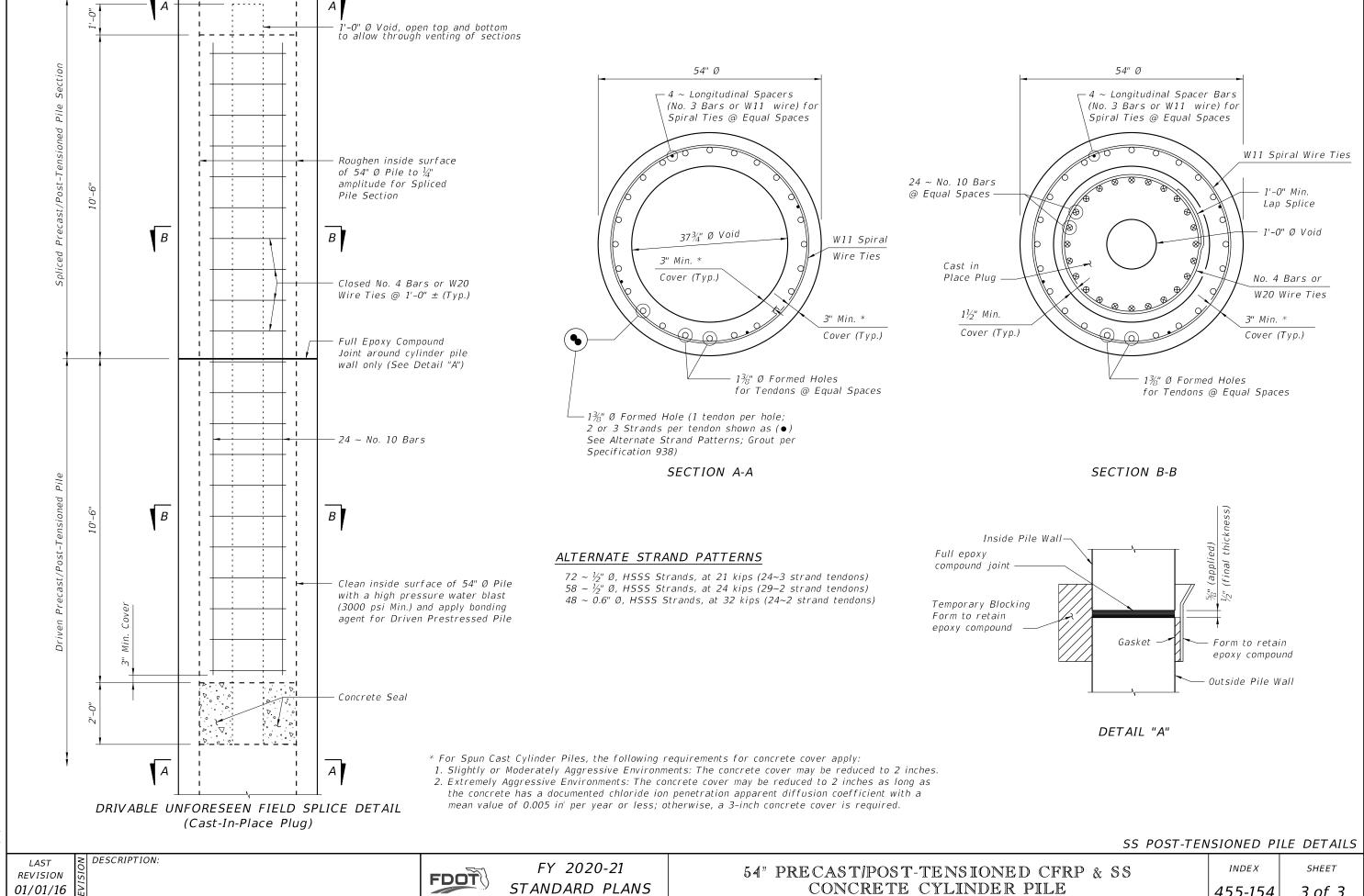
FY 2020-21 STANDARD PLANS

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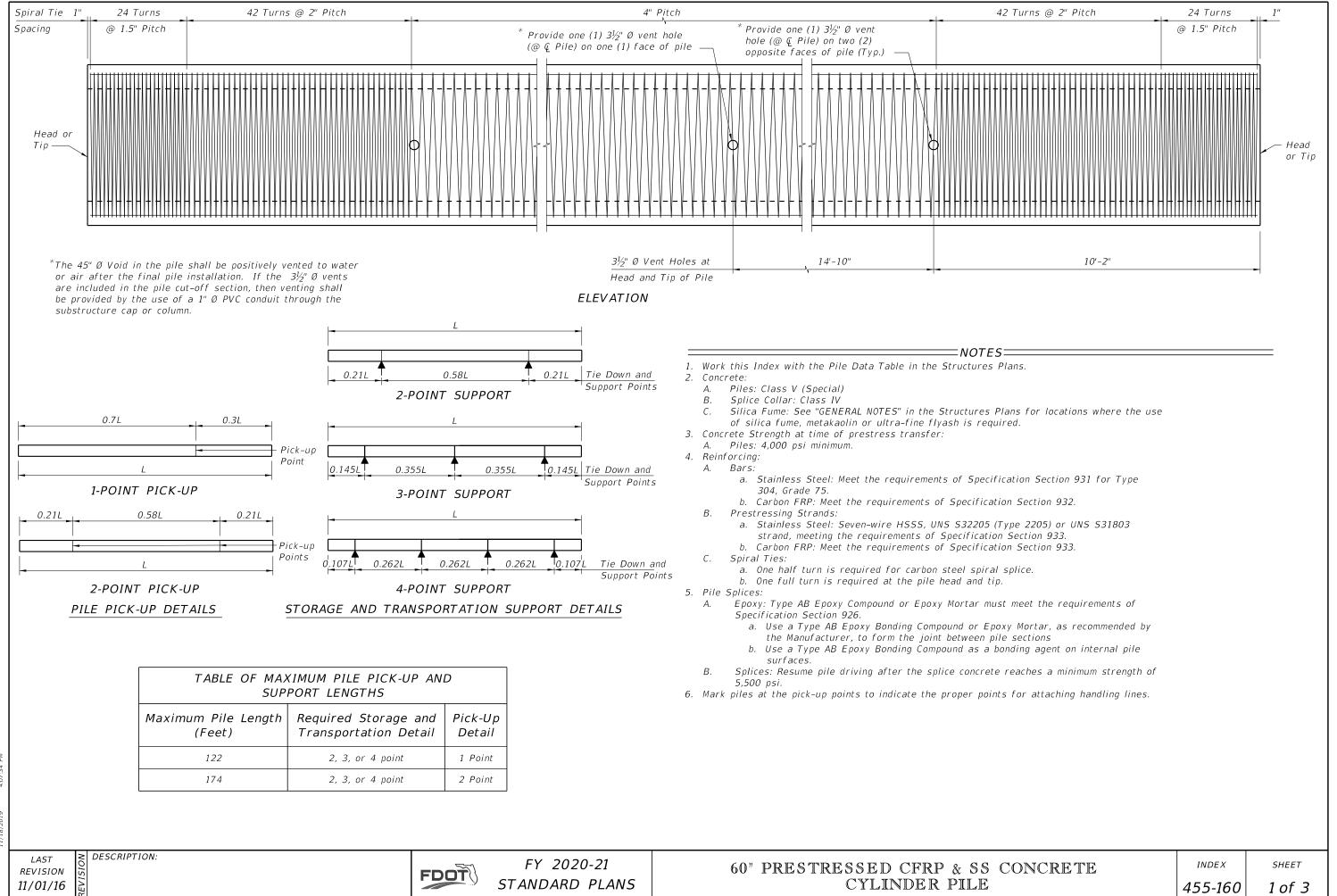


01/02/2011

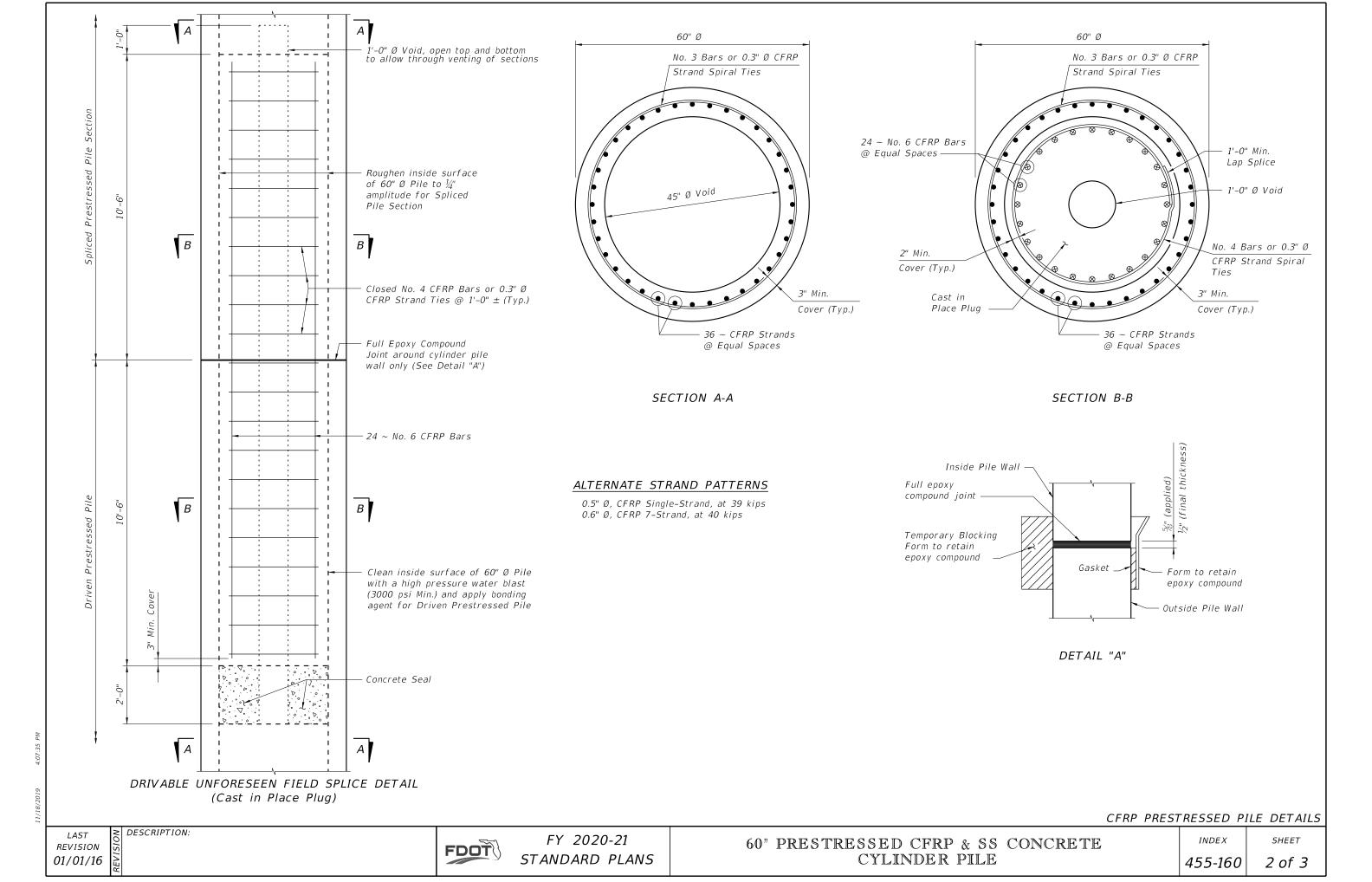


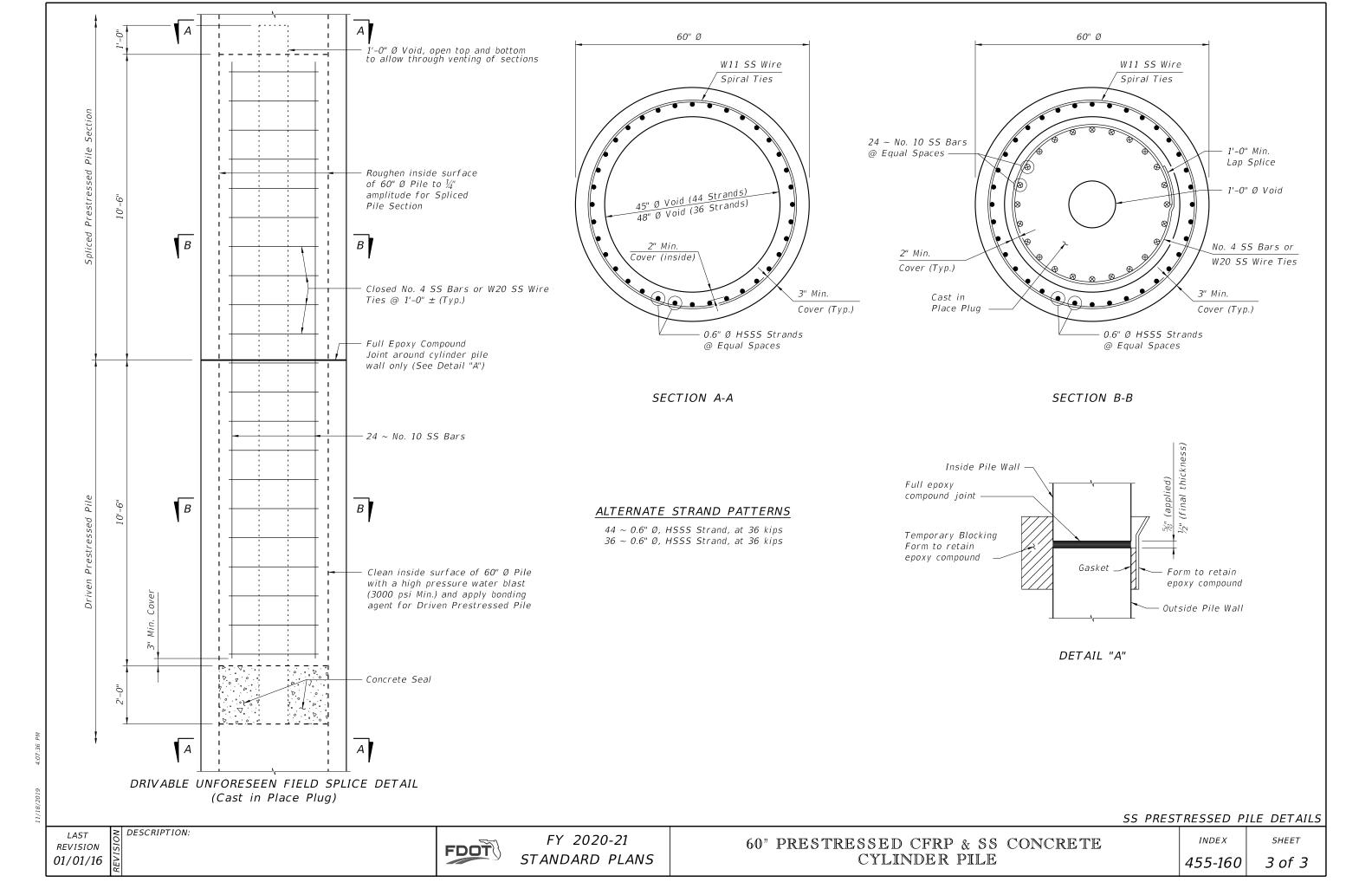


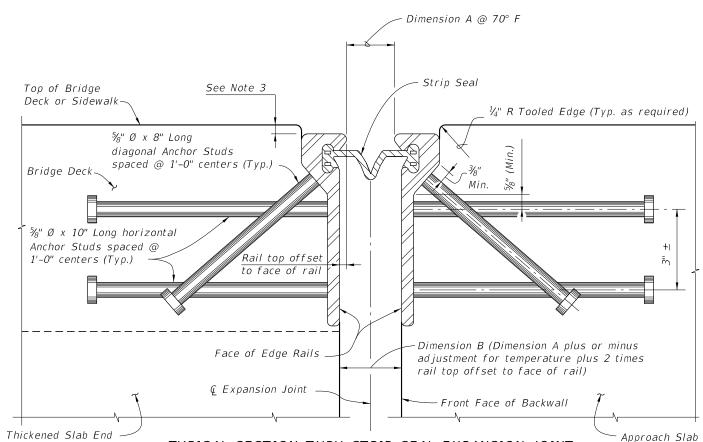
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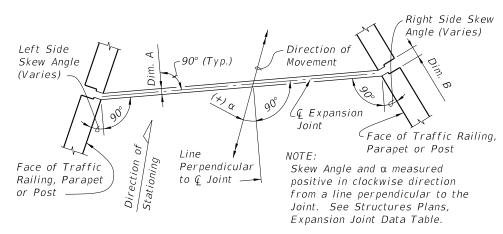
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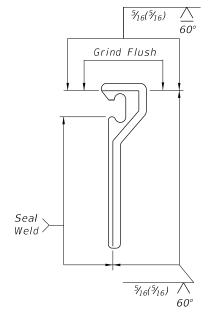




TYPICAL SECTION THRU STRIP SEAL EXPANSION JOINT (Begin or End Concrete Girder Bridge shown, Intermediate Supports and Steel Girder Bridge similar. Reinforcing Steel and Girder details not shown for clarity.)



MOVEMENT SCHEMATIC



SHOP SPLICE DETAIL

GENERAL NOTES:

- 1. Furnish Strip Seal Expansion Joint Systems in accordance with Specification Section 458.
- 2. Shape of Edge Rail shown is representative, minor variations depending on manufacturer
- 3. Recess the Edge Rail below the concrete surface in accordance with Specification Section 458.
- 4. Refer to the Expansion Joint Data Table in the Structures Plans for joint movement and Dimension A.
- 5. Refer to Specification Section 458 for installation and fabrication requirements.

REVISION 11/01/19

DESCRIPTION:

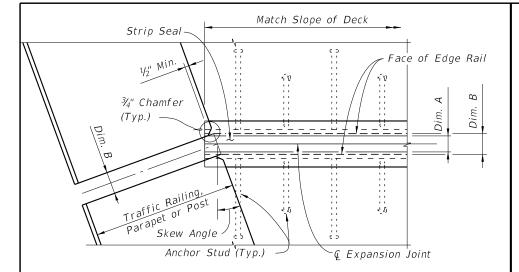
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FY 2020-21 STANDARD PLANS EXPANSION JOINT SYSTEM -STRIP SEAL

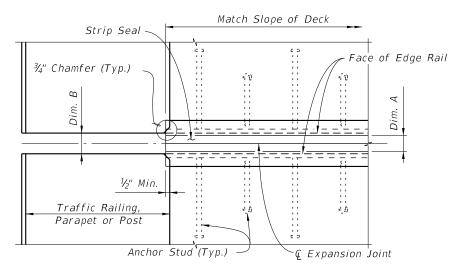
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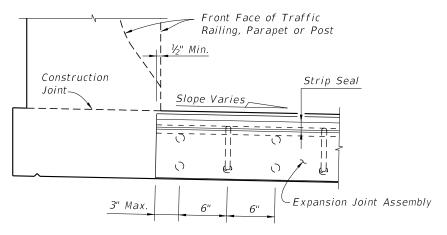
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PARTIAL PLAN VIEW OF SKEWED JOINTS



PARTIAL PLAN VIEW OF NONSKEWED JOINTS

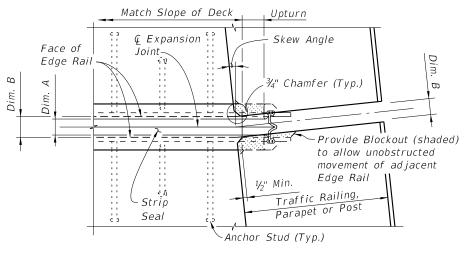


PARTIAL SECTION ALONG G JOINT

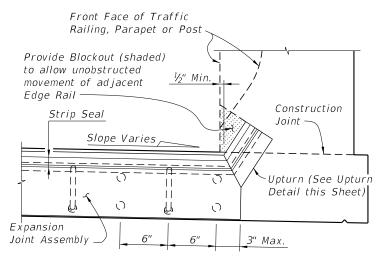
JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPE ≥ 1% (Sidewalk Cover Plate where applicable not shown for clarity)

Face of Edge Rail Ď ¾" Chamfer (Typ.) Turn Angle = Skew Angle/2 Skew Angle Match Slope of Deck

PARTIAL PLAN VIEW OF JOINTS SKEWED GREATER THAN 6°

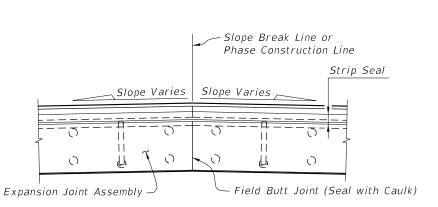


PARTIAL PLAN VIEW OF NONSKEWED JOINTS & JOINTS SKEWED 6° OR LESS

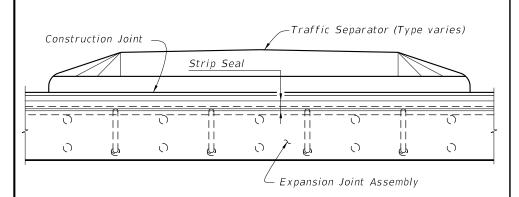


PARTIAL SECTION ALONG Q JOINT

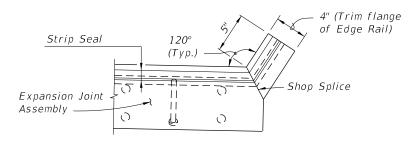
JOINT TREATMENT AT LOW SIDE OF DECK & HIGH SIDE OF DECK WITH SLOPE < 1% (Sidewalk Cover Plate where applicable not shown for clarity)



PARTIAL SECTION ALONG G JOINT AT FIELD BUTT JOINT LOCATION (CROWNED DECK OR SLAB SHOWN)



PARTIAL SECTION ALONG Q JOINT THRU TRAFFIC SEPARATOR



UPTURN DETAIL (TYPICAL AT TRAFFIC BARRIERS AND PARAPETS)

REVISION 11/01/19

DESCRIPTION:

FDOT

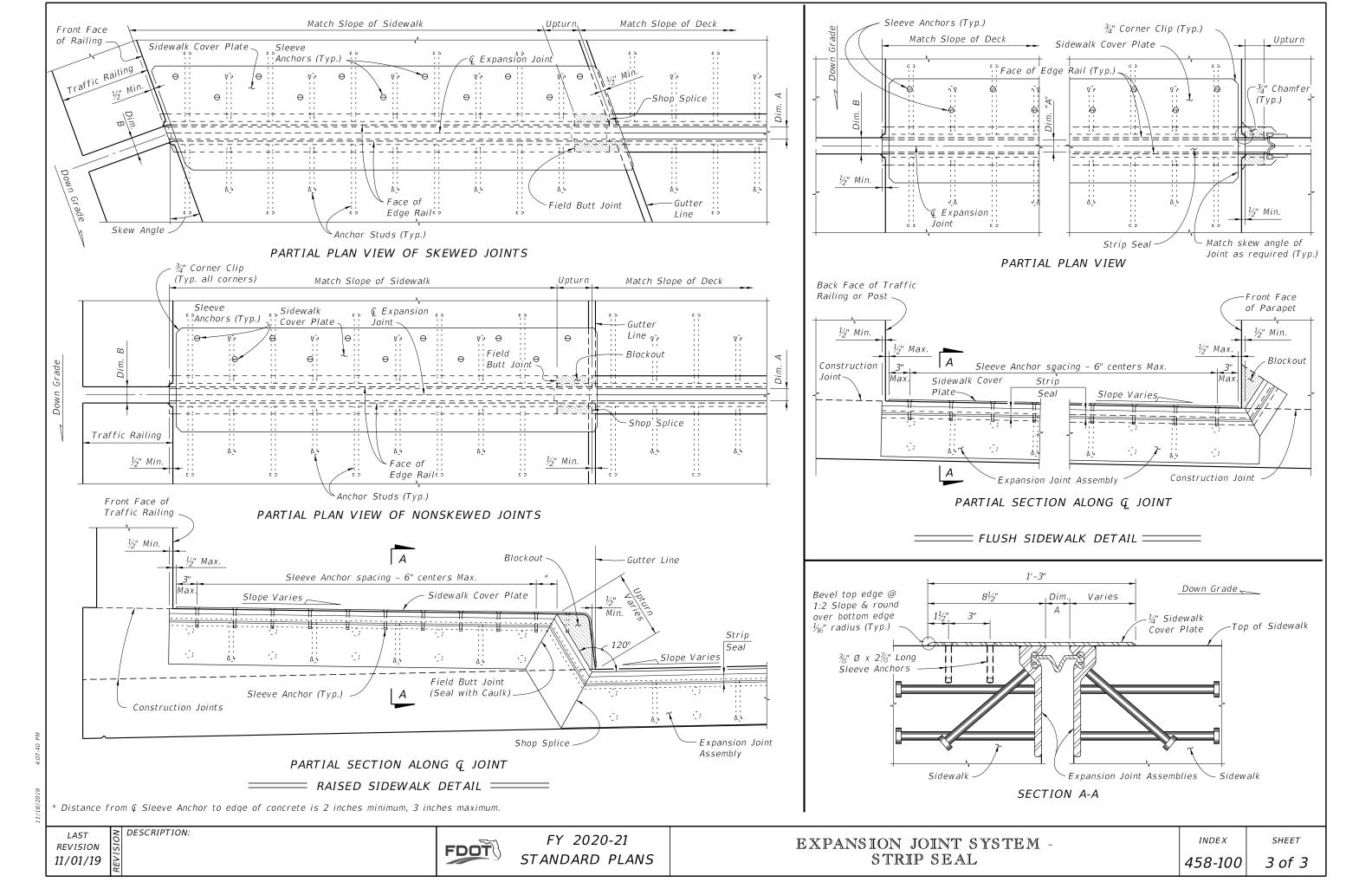
FY 2020-21 STANDARD PLANS STRIP SEAL

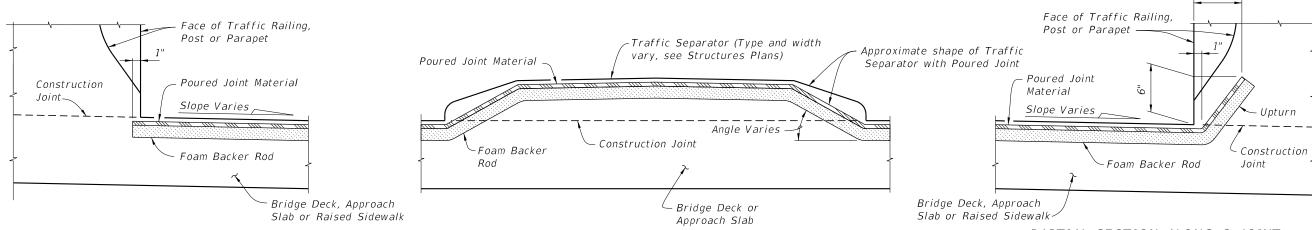
EXPANSION JOINT SYSTEM -

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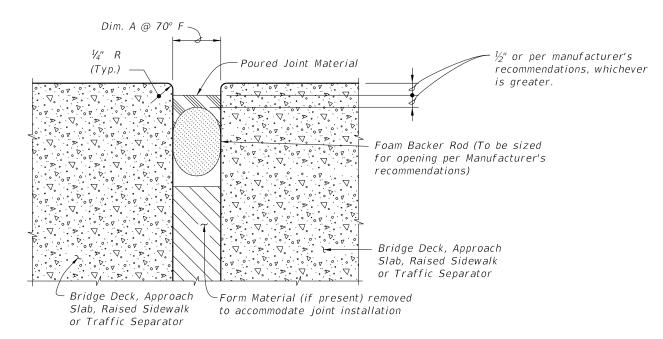




PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT HIGH SIDE OF DECK WITH SLOPES 1% OR GREATER

PARTIAL SECTION ALONG Q JOINT, JOINT TREATMENT AT TRAFFIC SEPARATOR

PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT LOW SIDE OF DECK OR HIGH SIDE OF DECK WITH SLOPES < 1%



TYPICAL SECTION THRU JOINT

GENERAL NOTES:

- 1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
- 2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.

REVISION 07/01/14

DESCRIPTION:



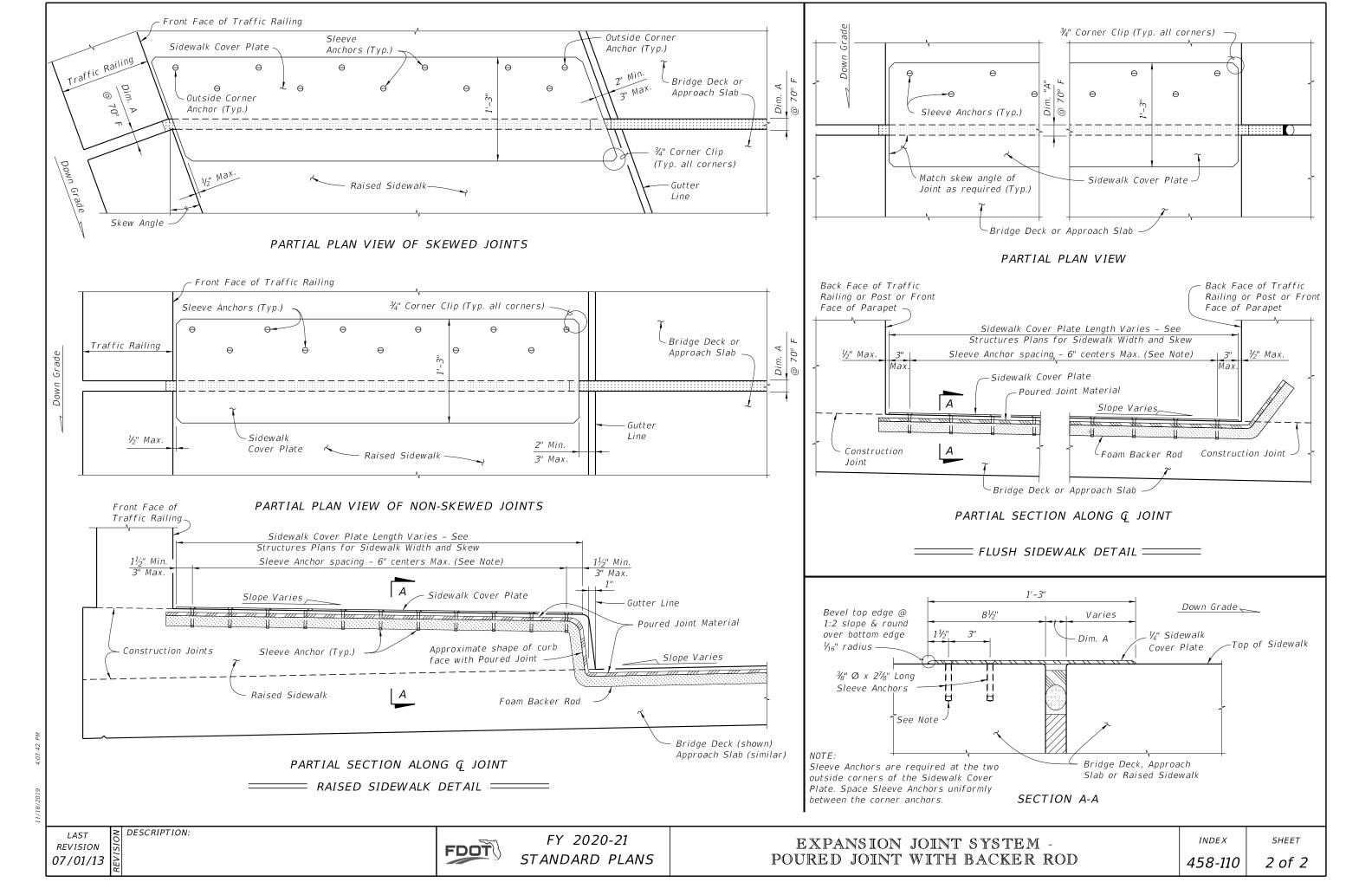
FY 2020-21 STANDARD PLANS

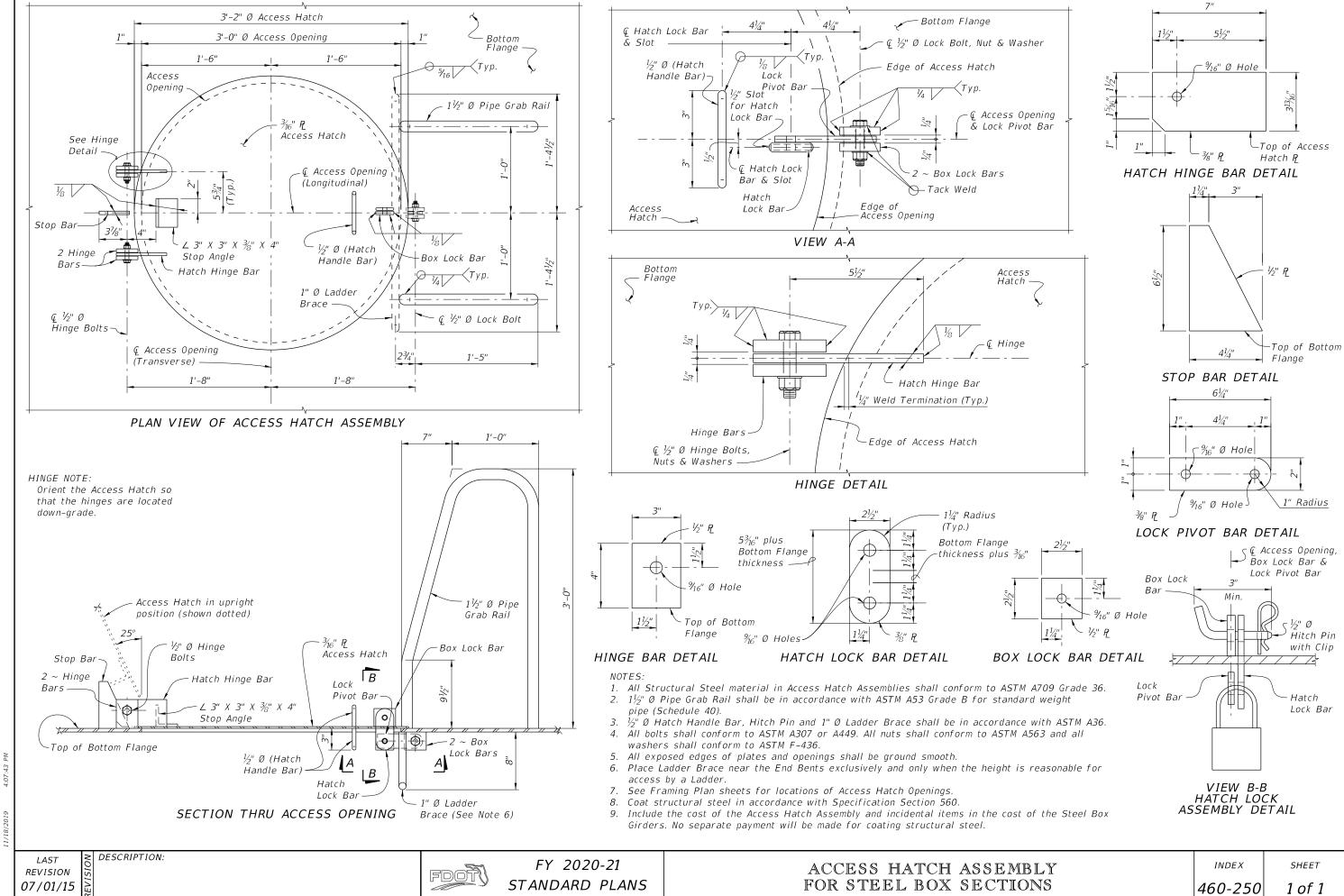
POURED JOINT WITH BACKER ROD

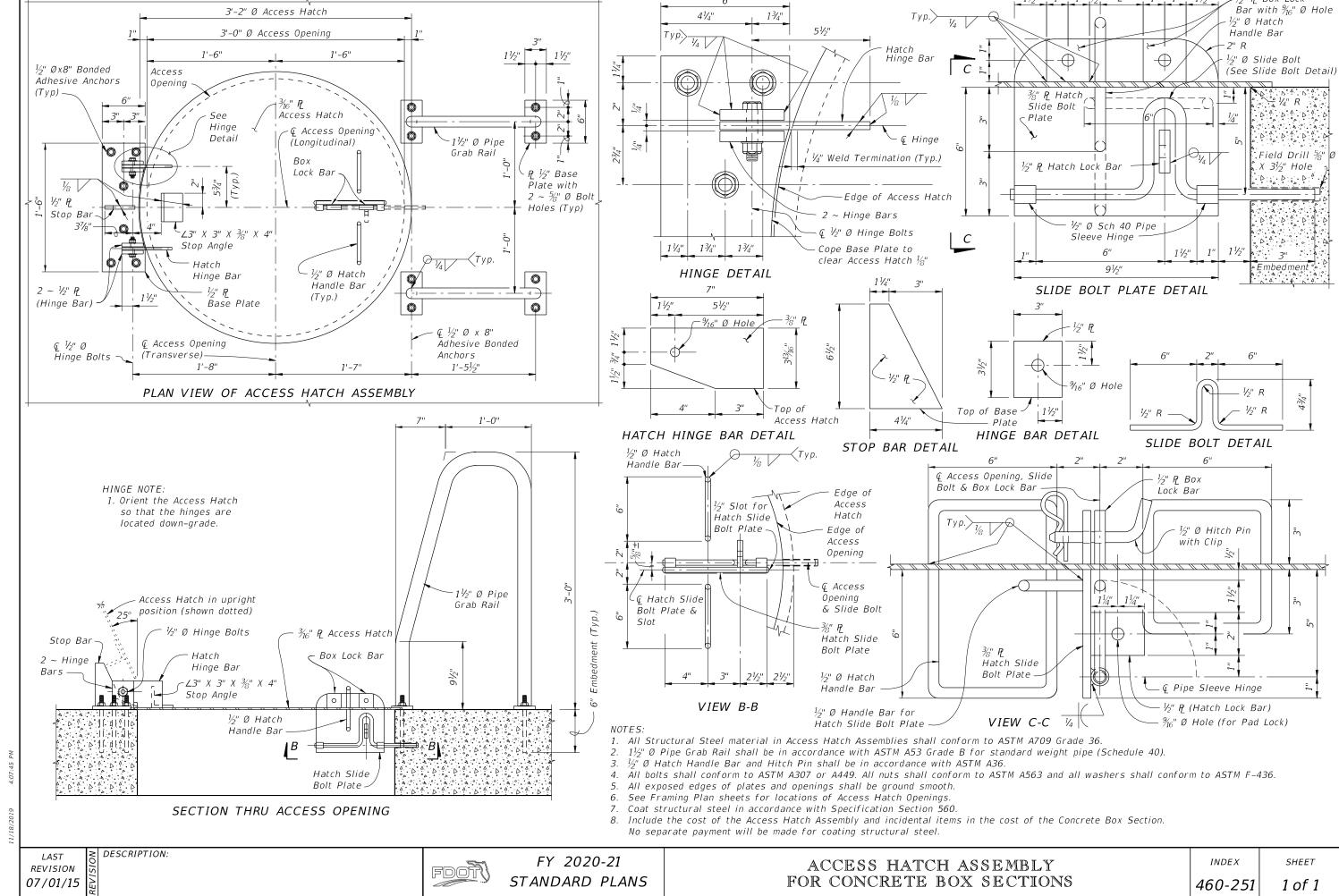
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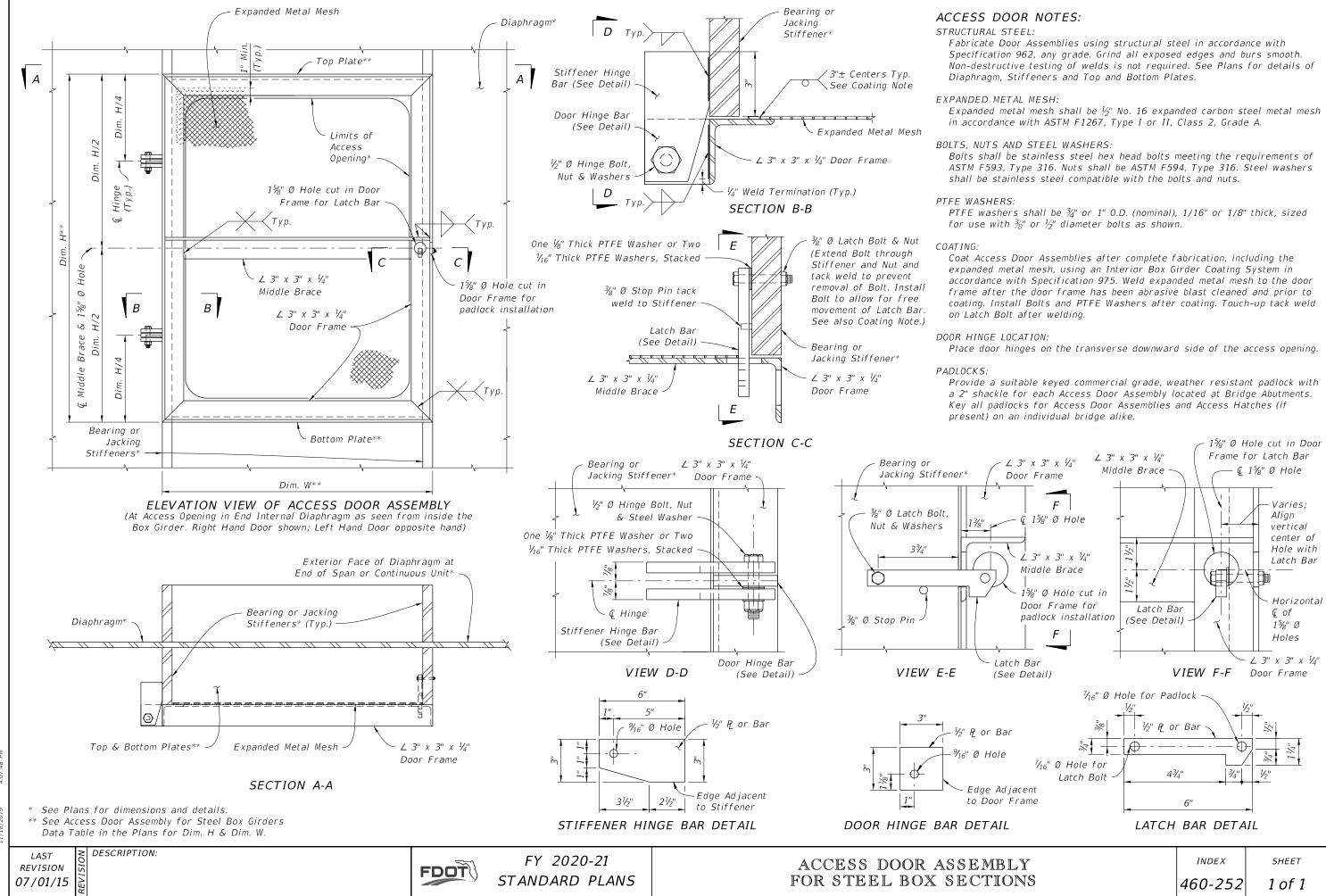
SHEET

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0,000

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be $3_4''$ by $2_7^{1/2}''$ slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM A449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for $\frac{1}{4}$ " Ø anchor bolts; 55,000 lbs. for the $\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the $\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1½" spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

BEARING PADS: Provide plain Neoprene pads with a durometer hardness of 60 or 70 and meeting the requirements of Specification Section 932, for ancillary structures.

ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

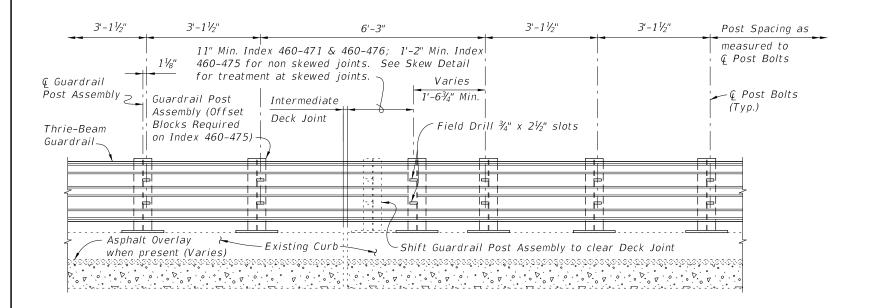
BARRIER DELINEATORS: Install Barrier Delineators at the top of the guardrail offset blocks in accordance with Specification Section 705. Match the Barrier Delineators color (white or yellow) to the near edgeline.

PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index 536-001 for details.

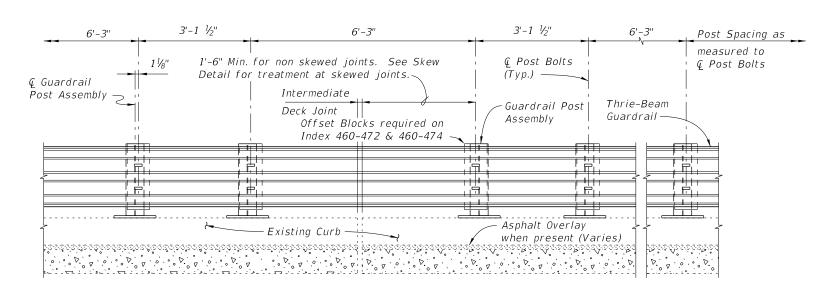
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

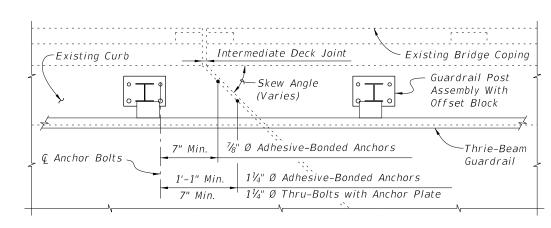
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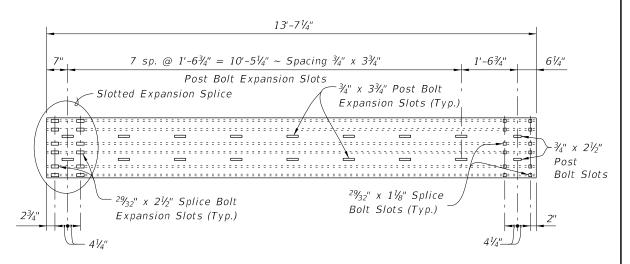
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL
FOR INDEX 460-471, 460-475 & 460-476



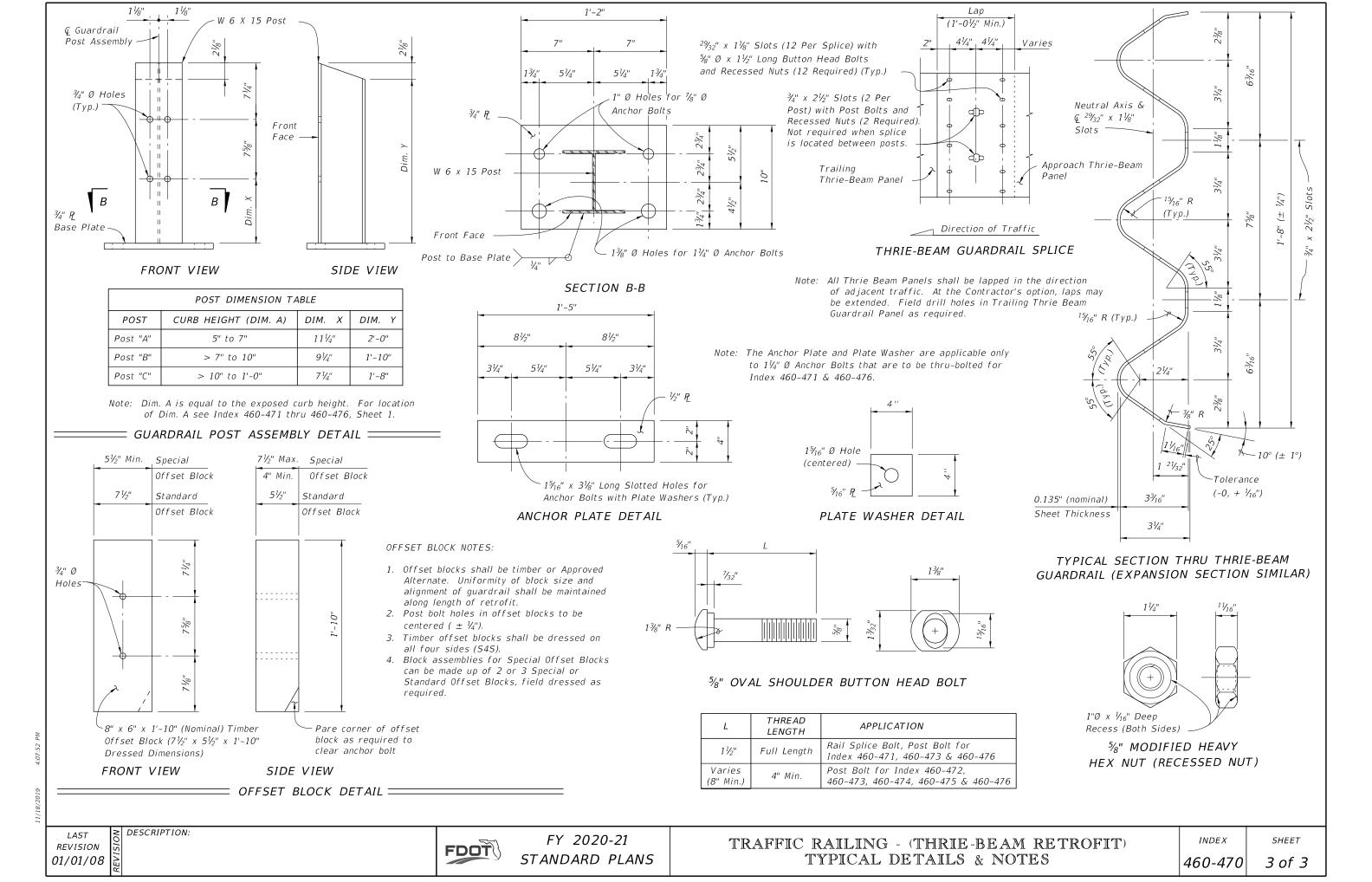
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL
FOR INDEX 460-472, 460-473 & 460-474

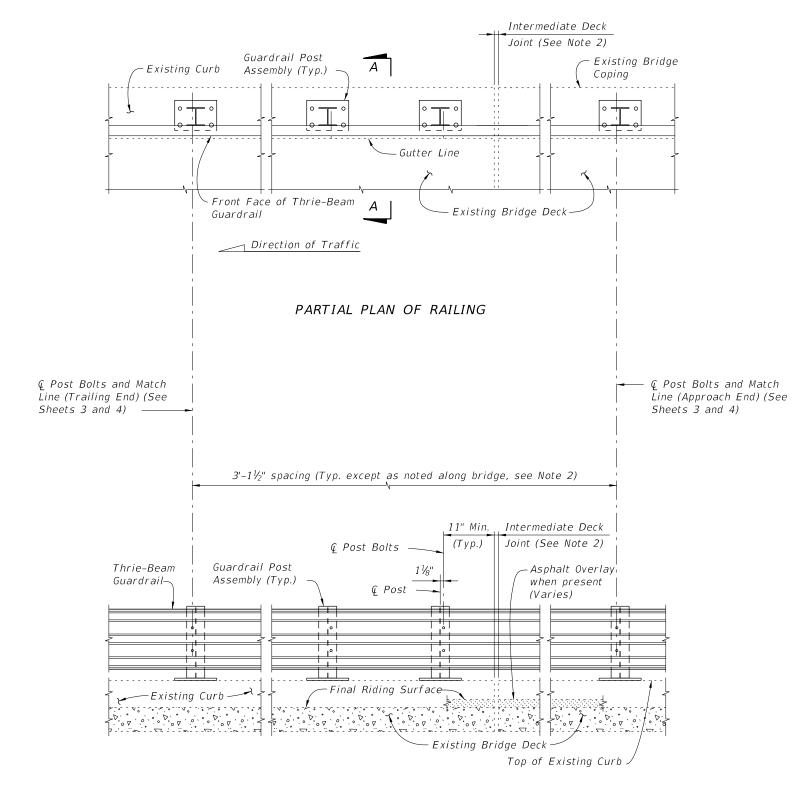


PARTIAL PLAN
INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

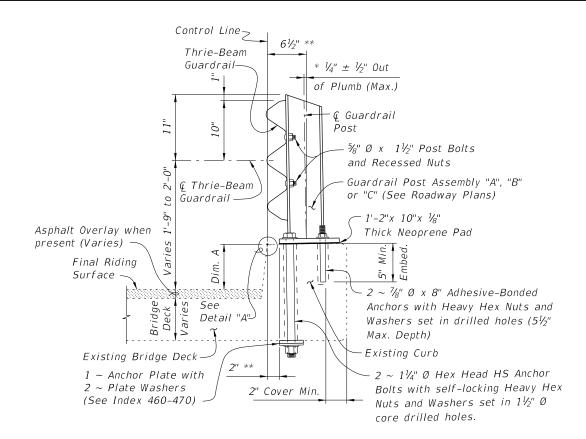
REVISION 01/01/08

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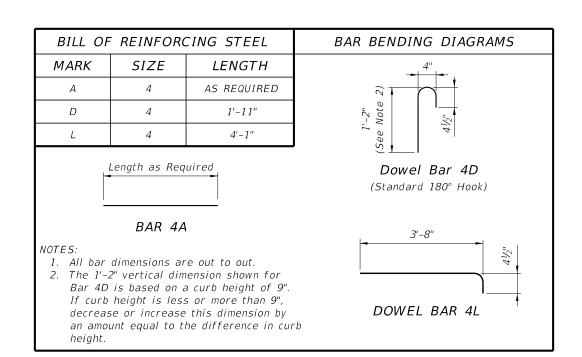
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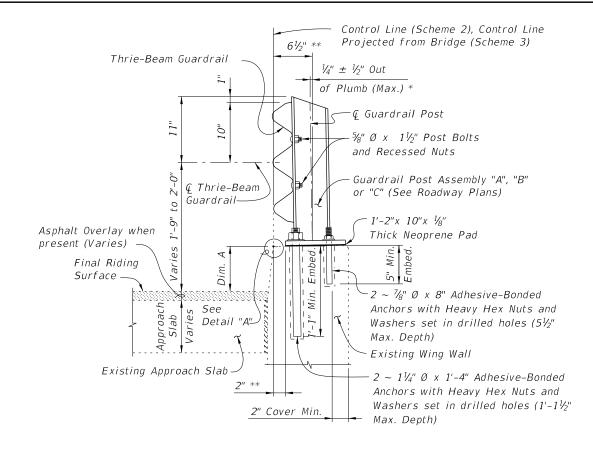
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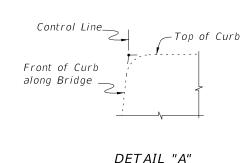
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

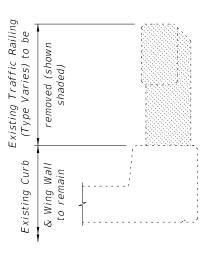




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

DESCRIPTION:

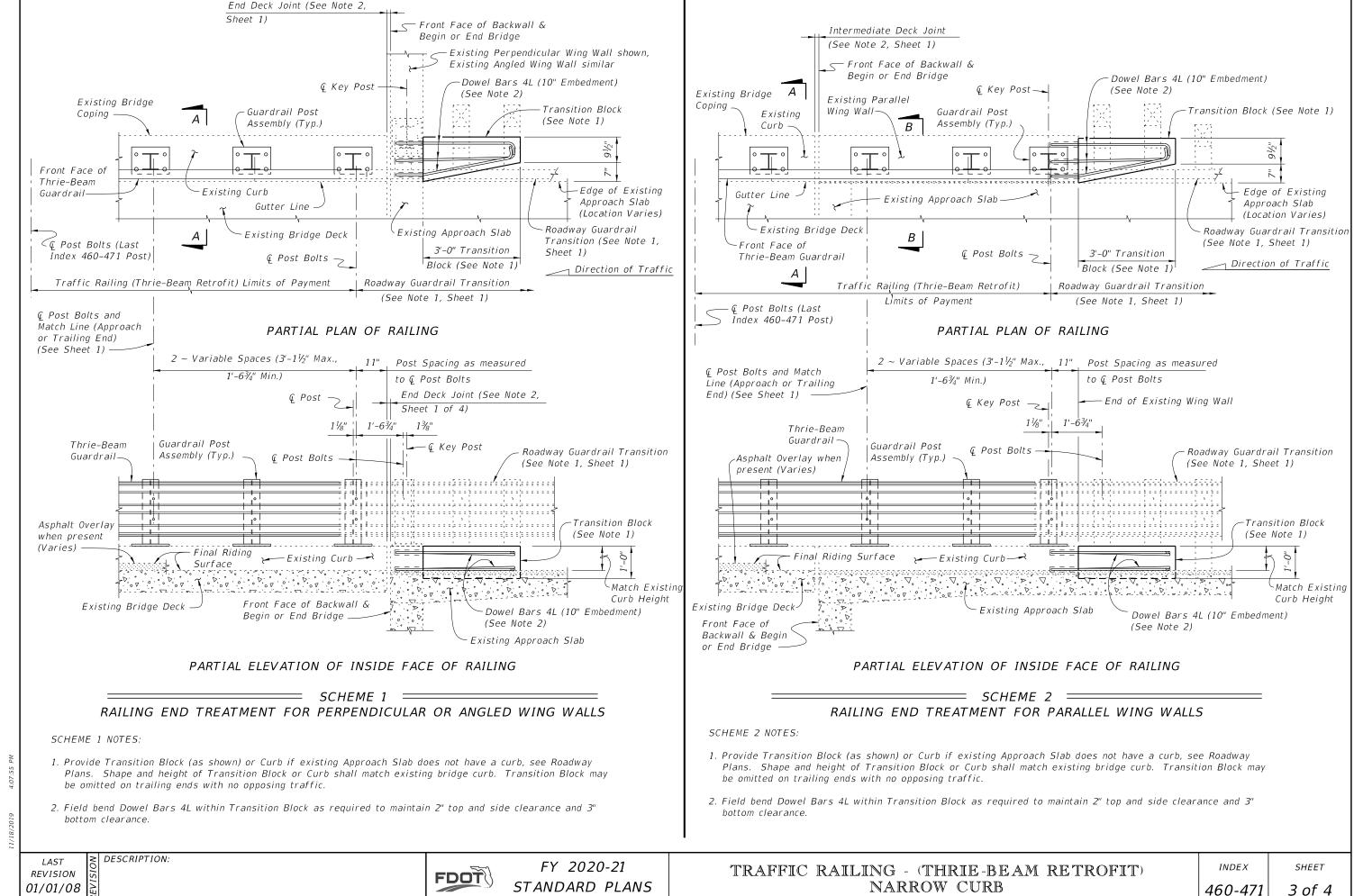
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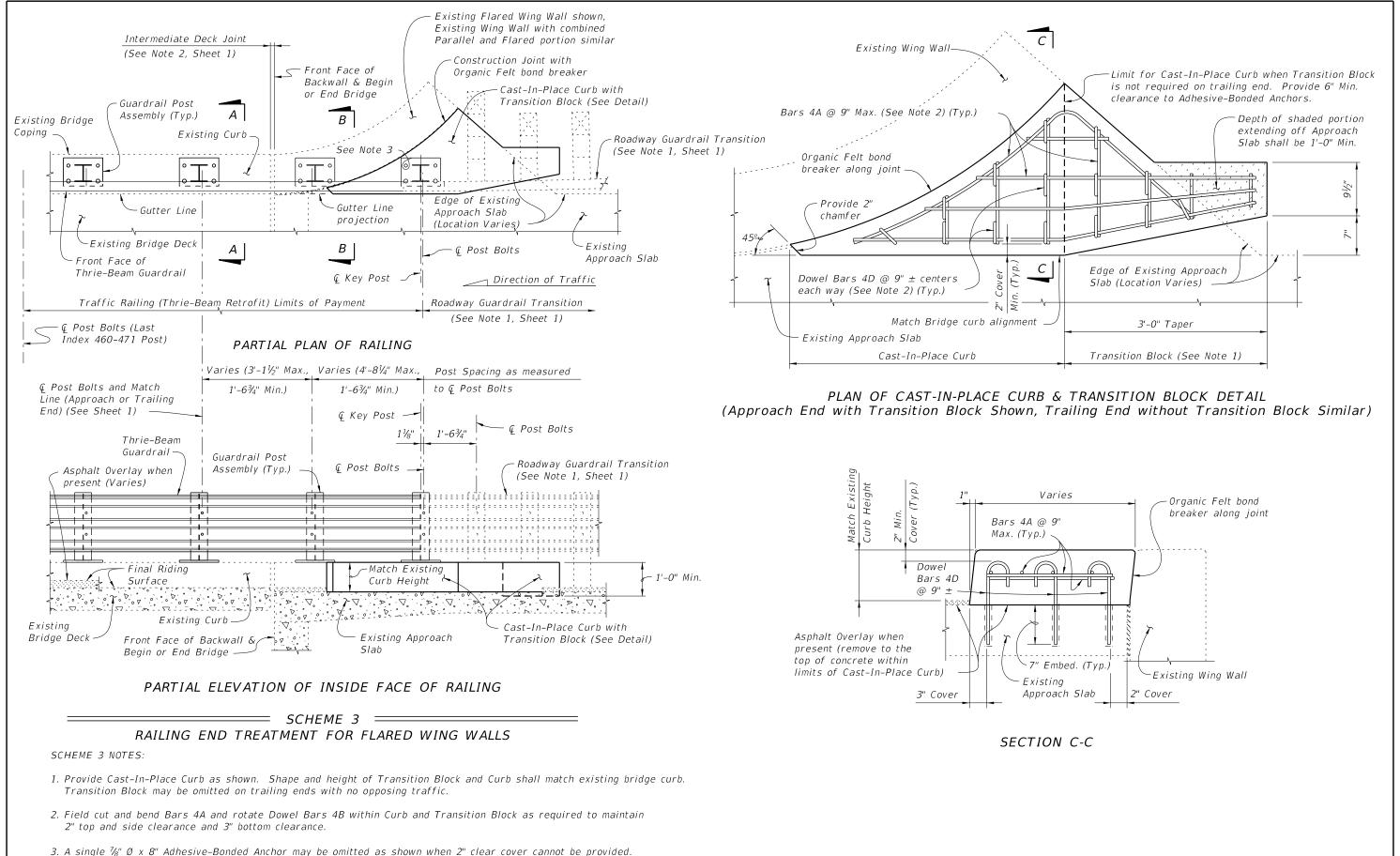
FY 2020-21 STANDARD PLANS TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

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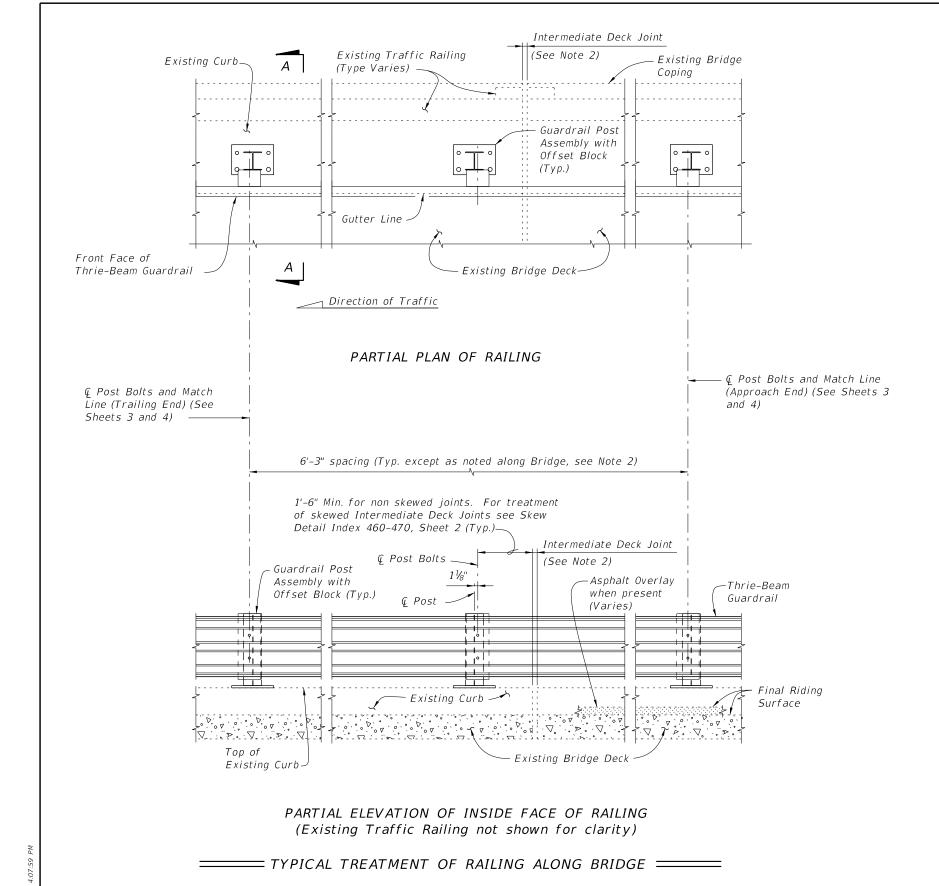




DESCRIPTION: LAST REVISION 11/01/16



SHEET



NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Shee

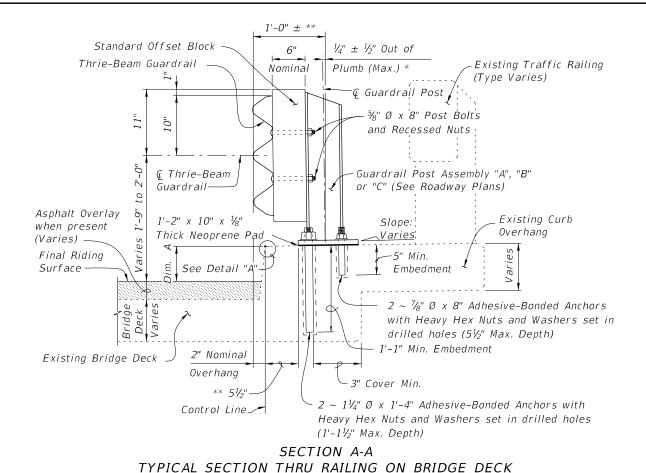
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

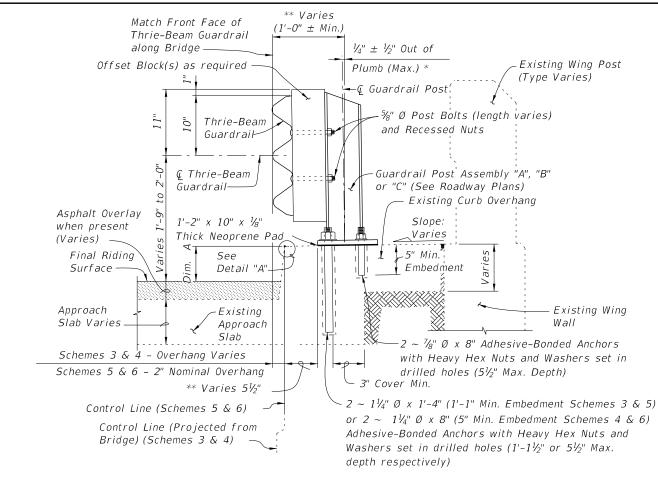
LAST REVISION 01/01/08

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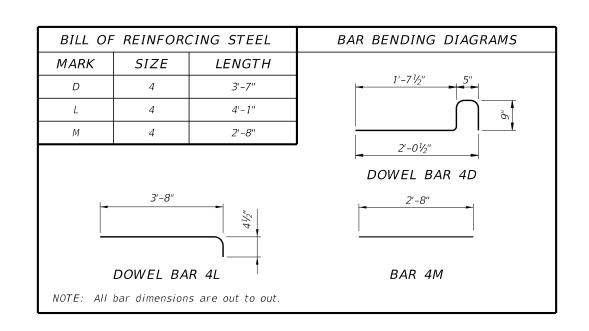
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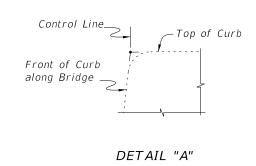


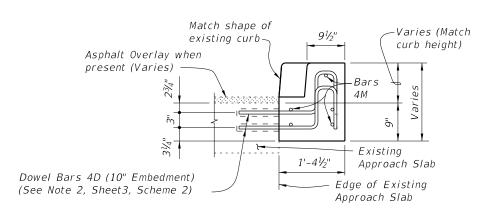
SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)



Shim with washers around Anchors as required to maintain tolerance.

Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 07/01/08

DESCRIPTION:

FDOT

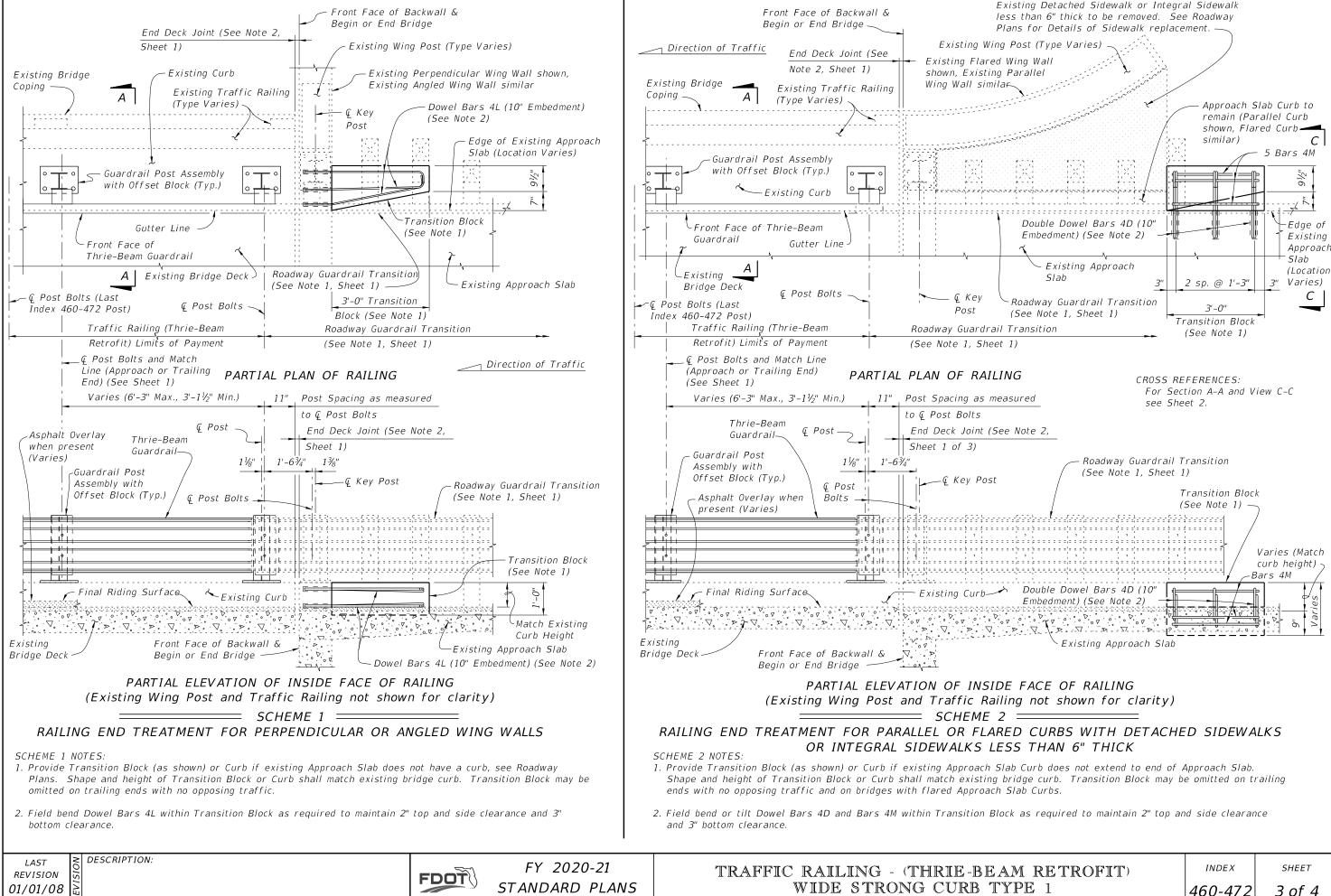
FY 2020-21 STANDARD PLANS

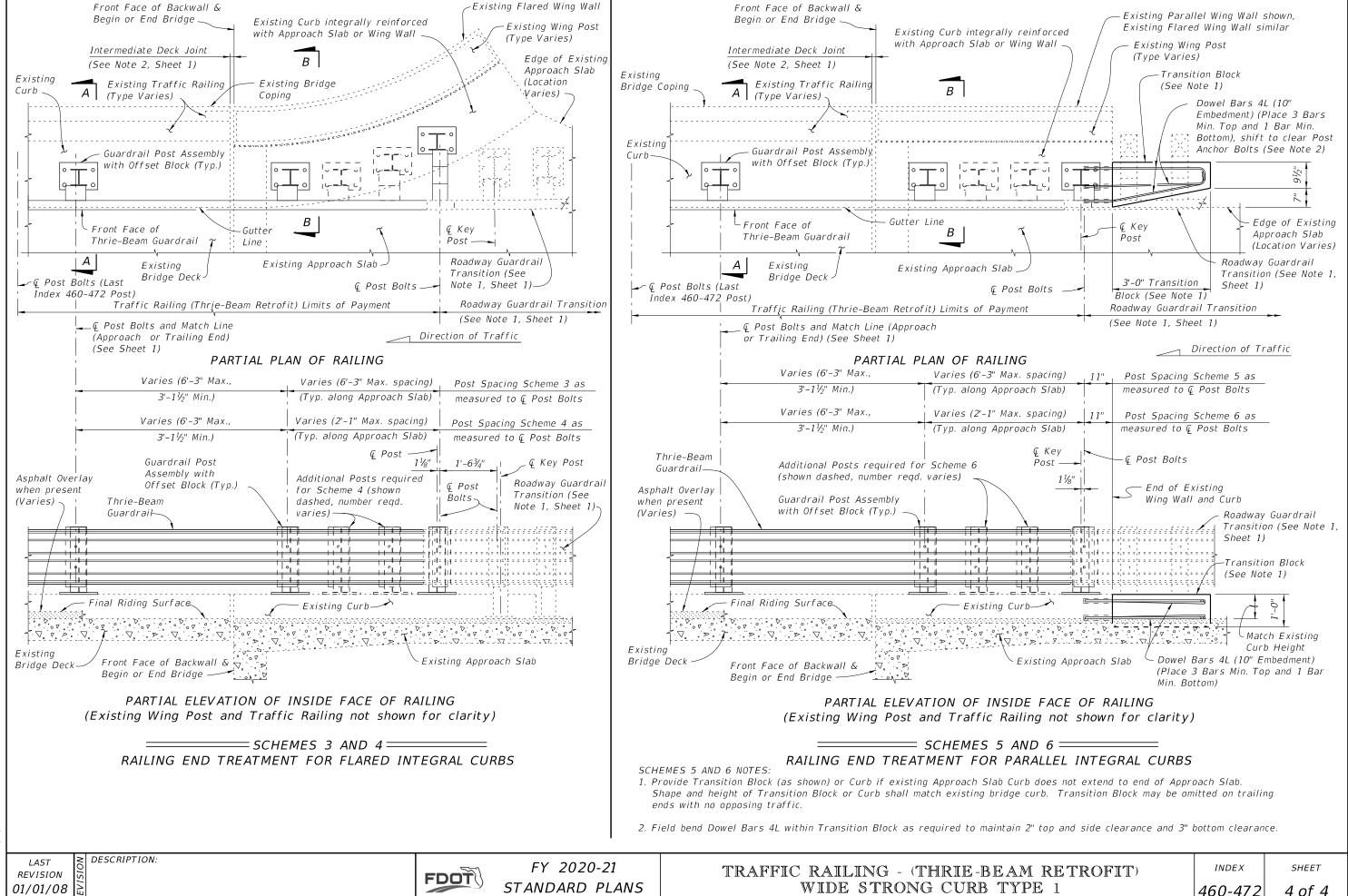
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 1

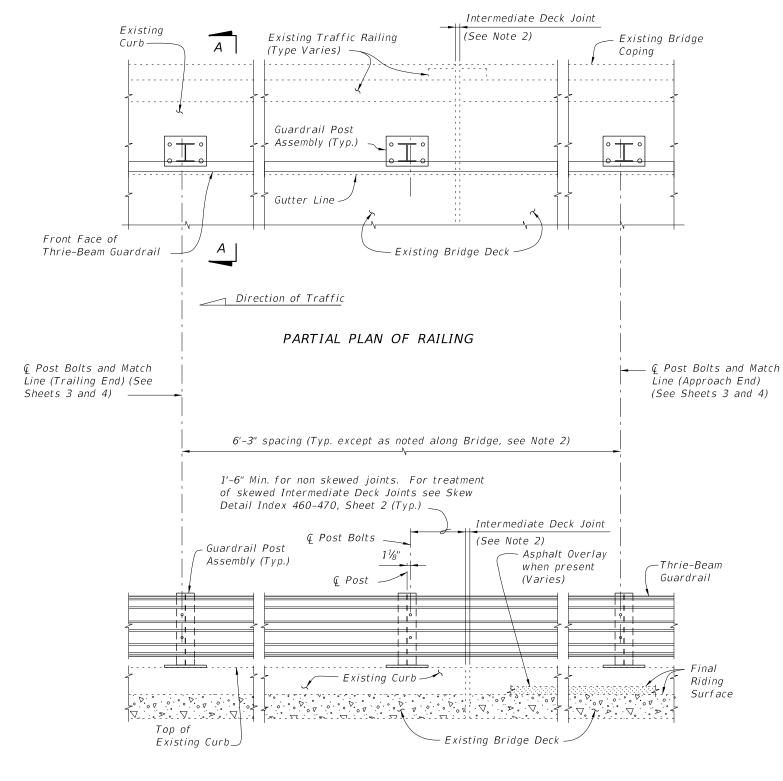
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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index 460-470.

LAST REVISION 01/01/08

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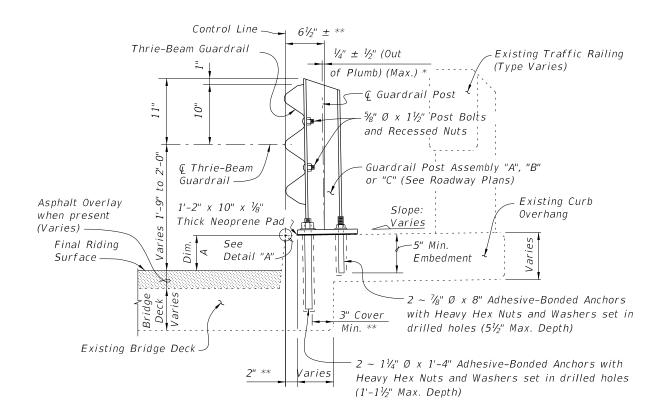


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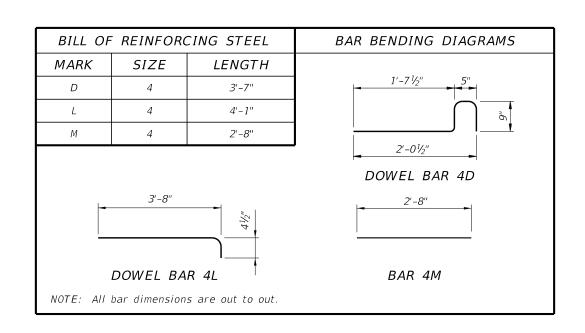
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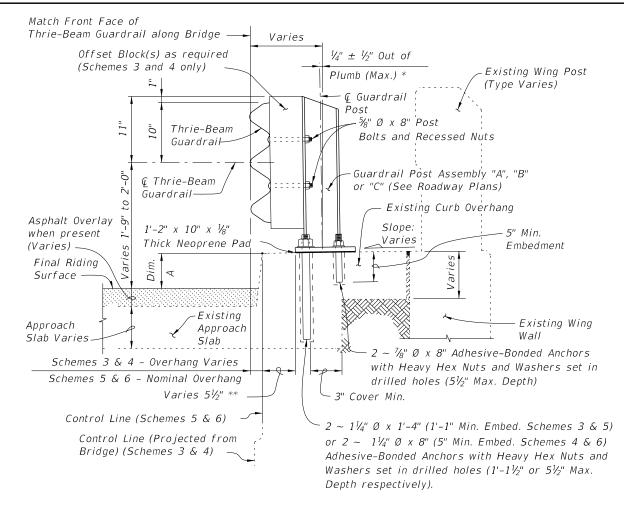
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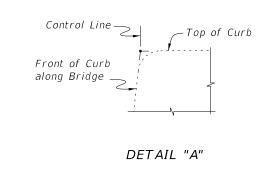
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

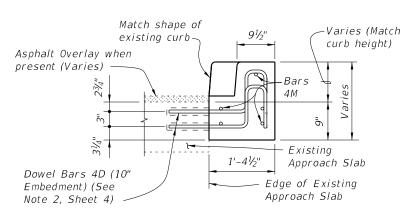




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- * Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- ** Offset may vary \pm 1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index 460-470.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

DESCRIPTION:

FDOT

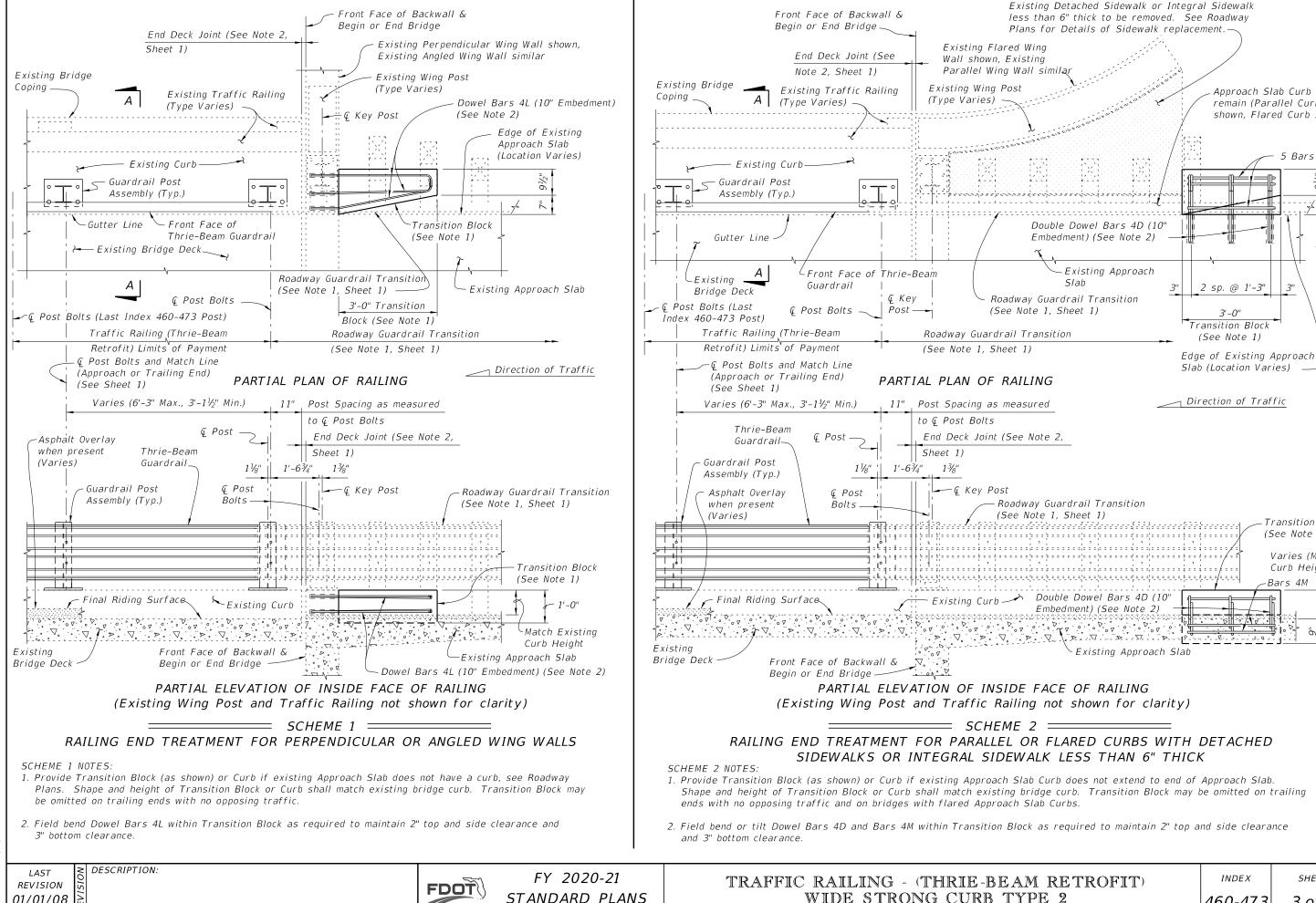
FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2

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WIDE STRONG CURB TYPE 2

INDEX 460-473

Approach Slab Curb to

shown, Flared Curb similar,

С

5 Bars 4M

Transition Block

Varies (Match Curb Height) -

(See Note 1)

-Bars 4M

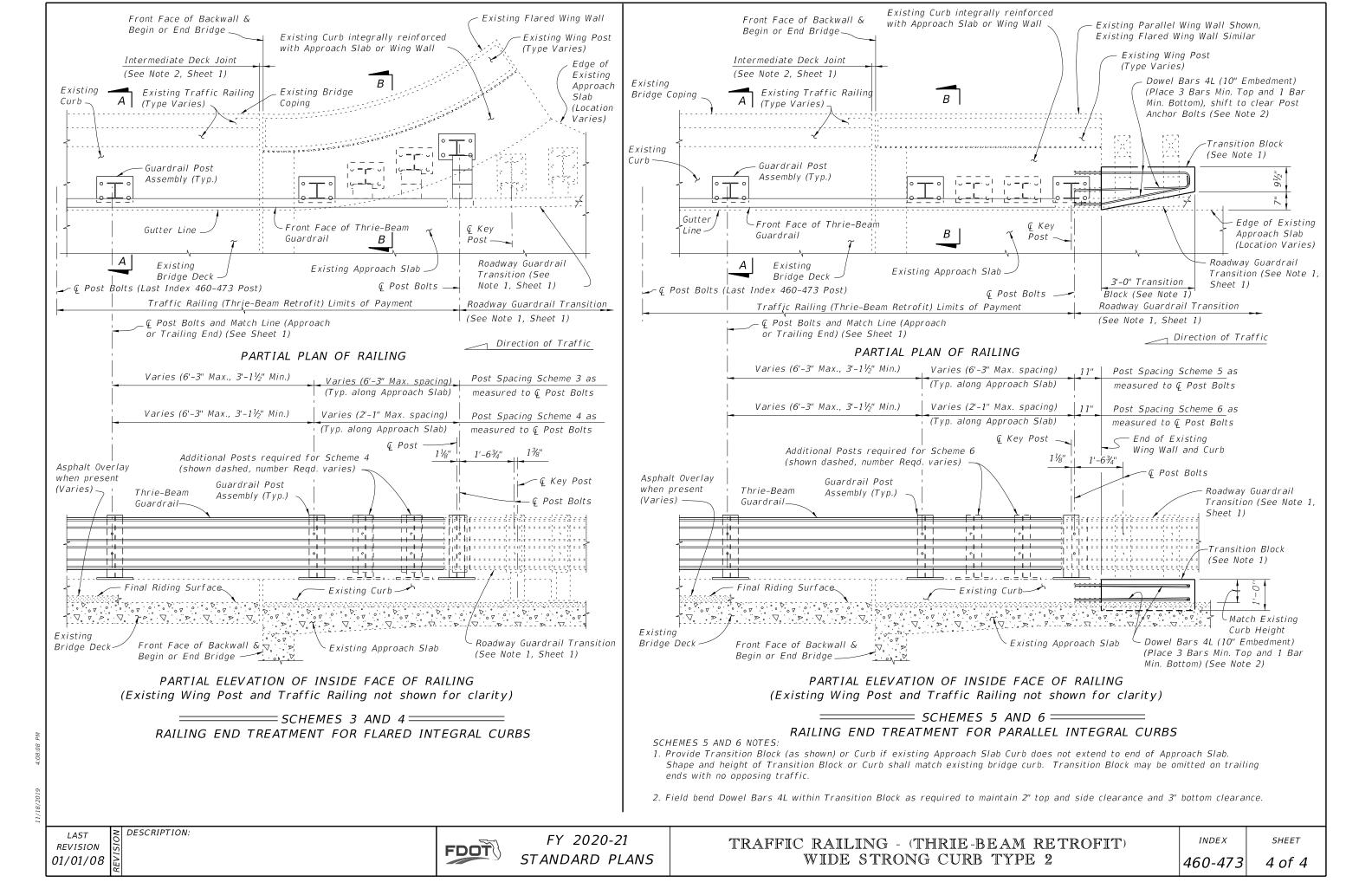
remain (Parallel Curb

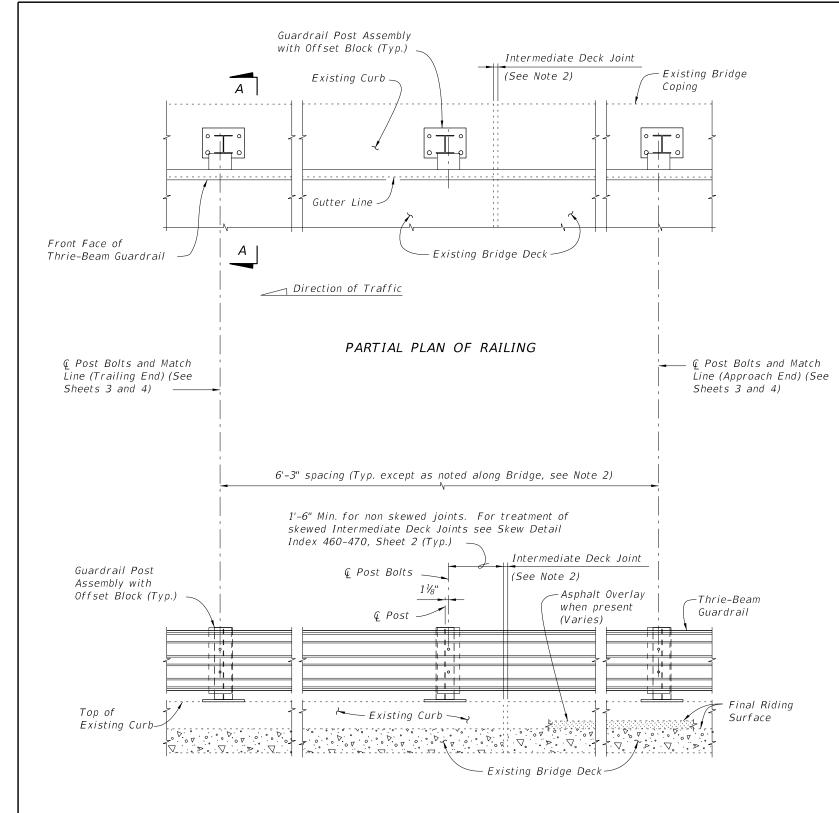
2 sp. @ 1'-3"

3'-0"

(See Note 1)

SHEET 3 of 4





PARTIAL ELEVATION OF INSIDE FACE OF RAILING

===== TYPICAL TREATMENT OF RAILING ALONG BRIDGE =======

NOTES:

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

REVISION 01/01/08

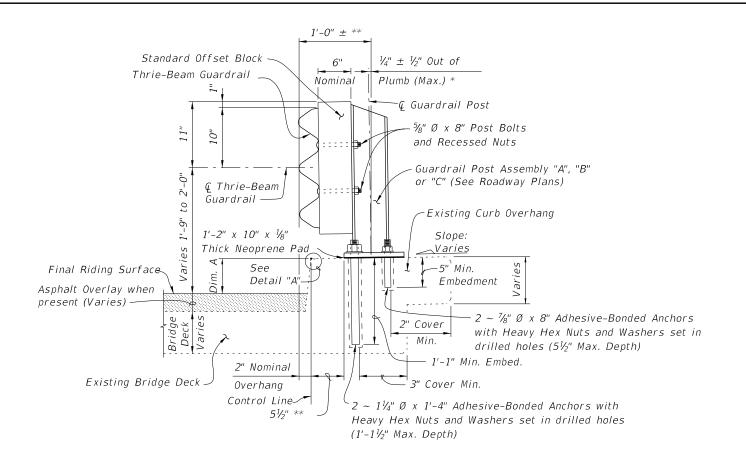
FDOT

INDEX

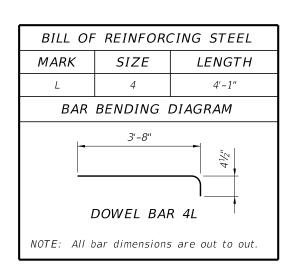
SHEET

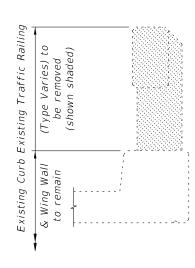
460-474 1 of 4

DESCRIPTION:

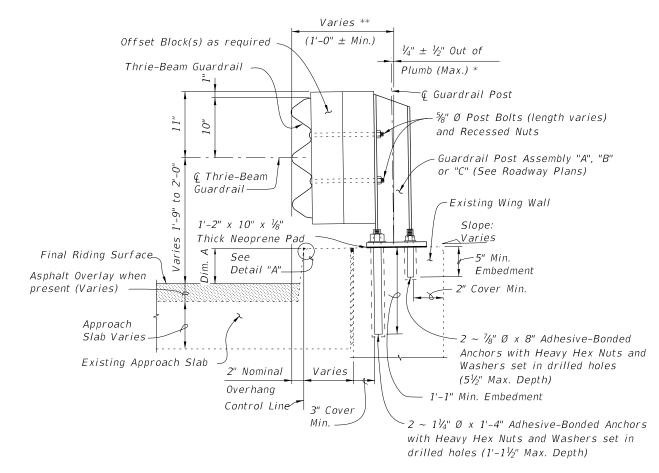


SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





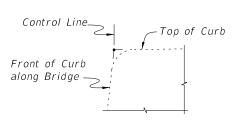
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION B-B (SCHEME 2) TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

 st Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

 ** Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



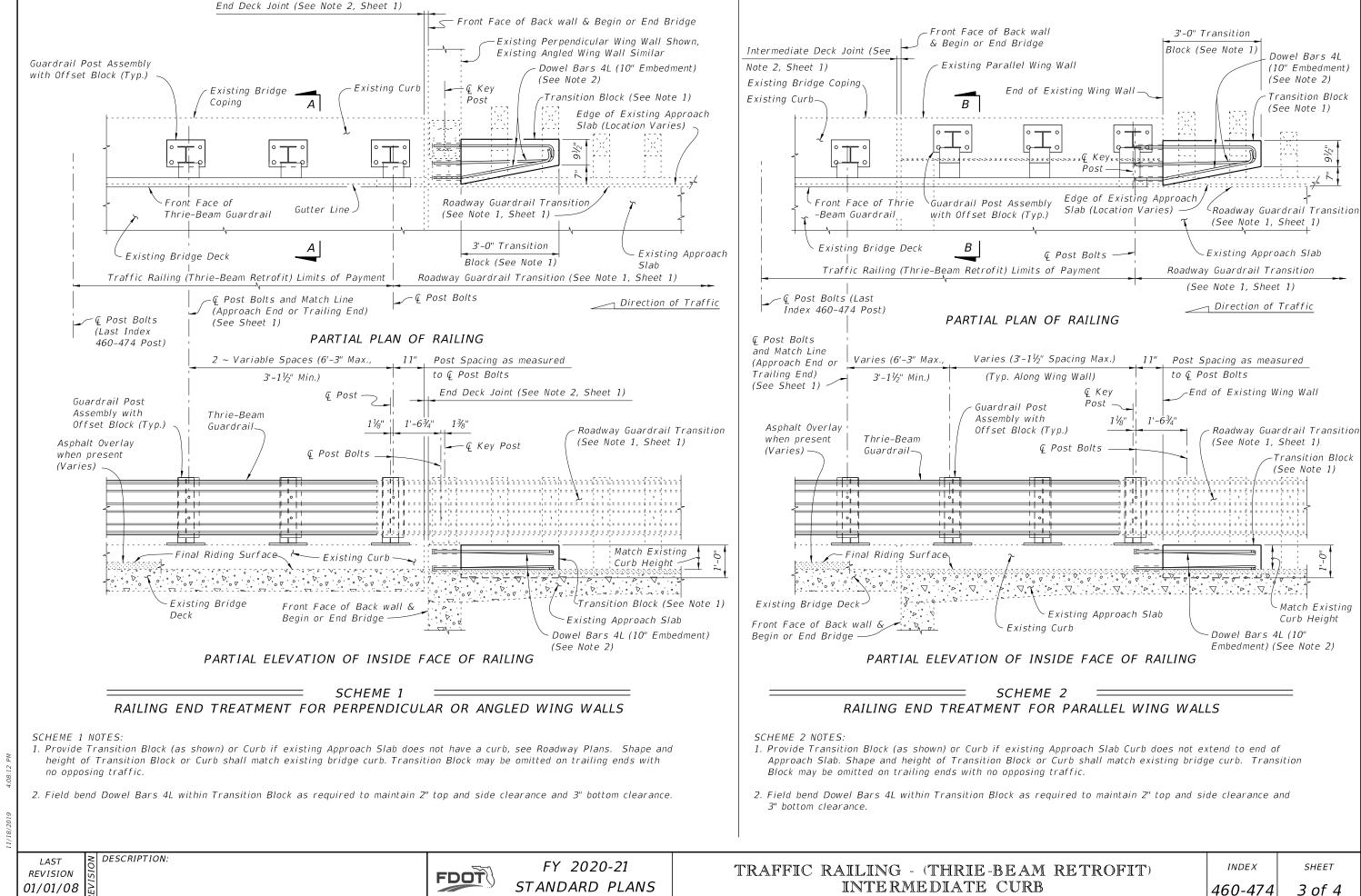
DETAIL "A"

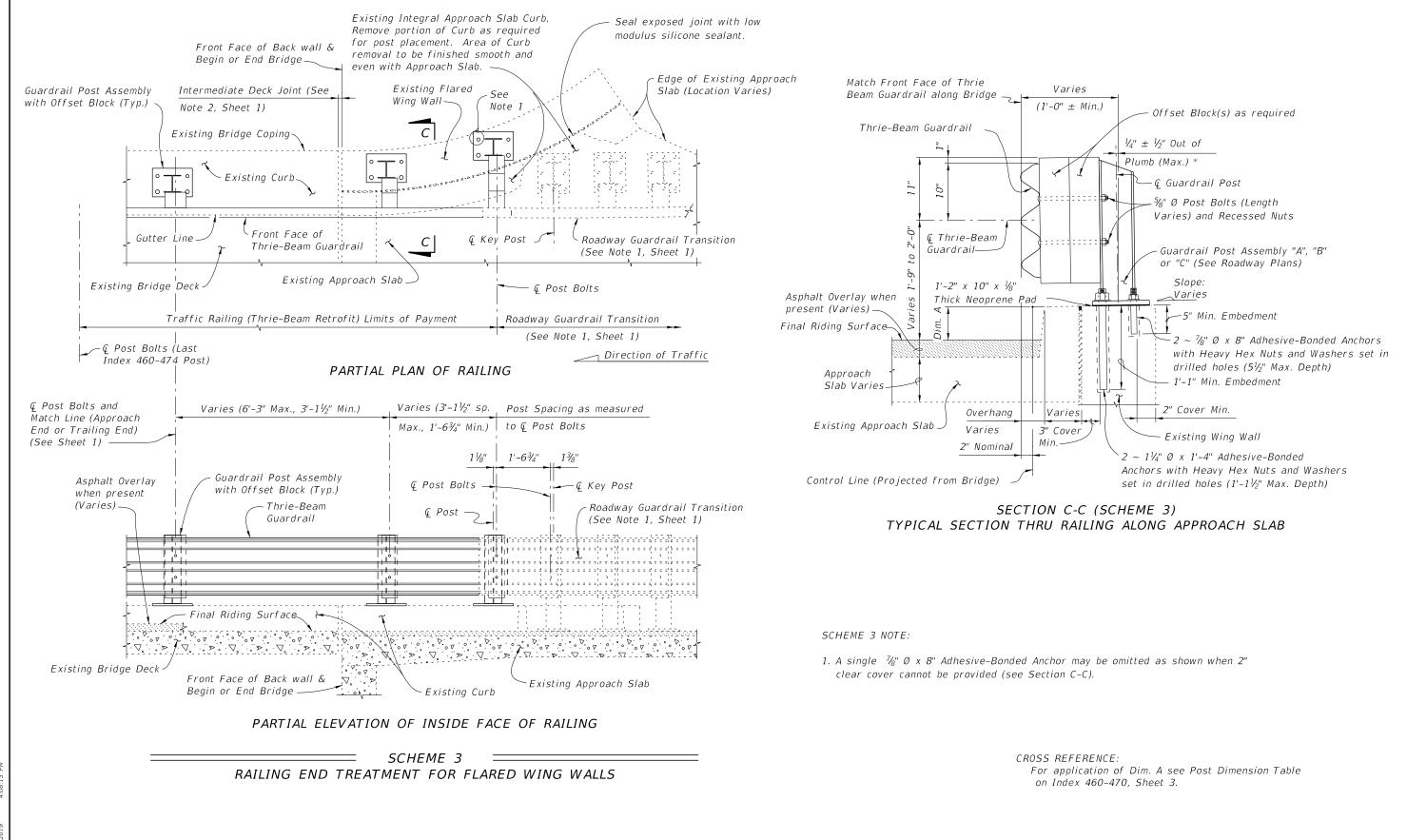
CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3. For location of Section B-B see Sheet 3 For application of Dim. A see Post Dimension Table on Index 460-470, Sheet 3.

DESCRIPTION:

SHEET



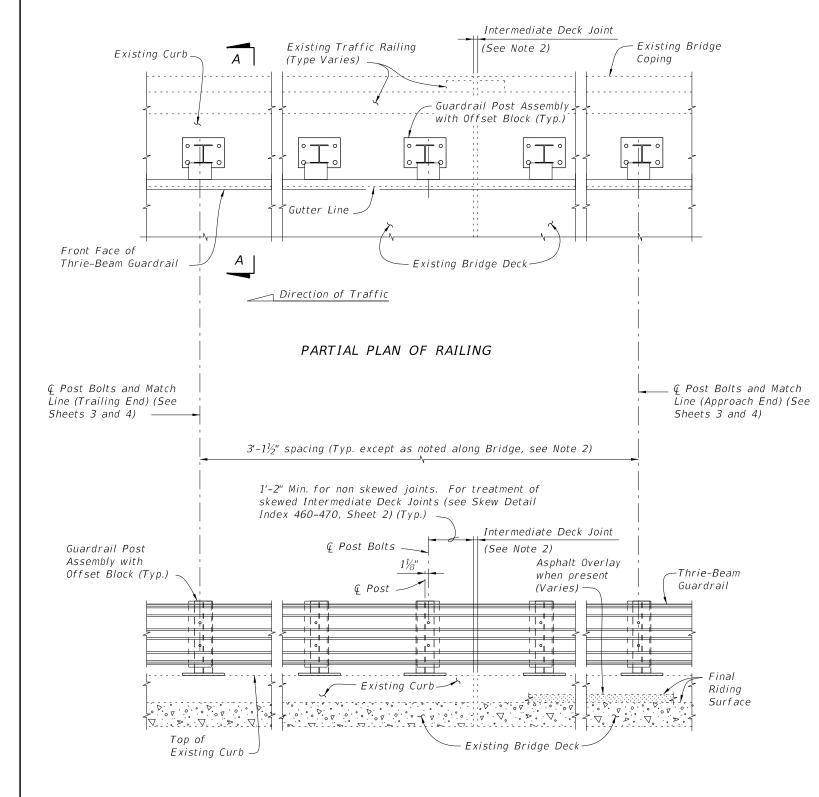


11/18/2019

LAST DESCRIPTION:
REVISION 5
07/01/09

FDOT

FY 2020-21 STANDARD PLANS



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

= TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

REVISION 01/01/08

FDOT

FY 2020-21 STANDARD PLANS

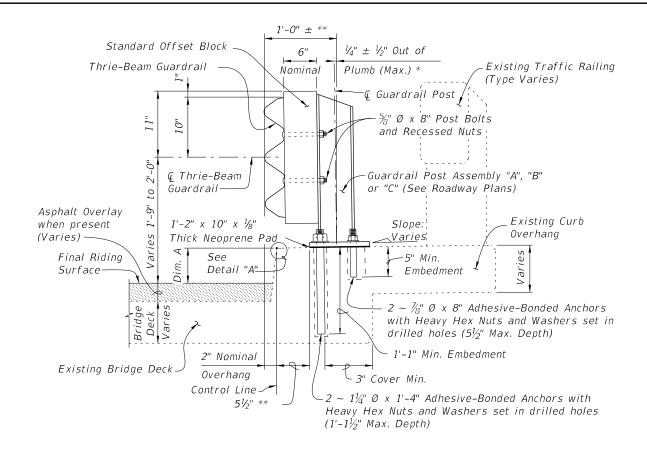
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

INDEX

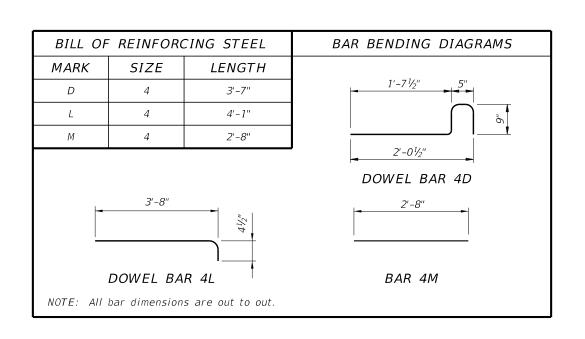
SHEET

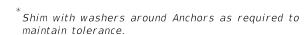
460-475 1 of 4

DESCRIPTION:



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

when present

Surface

Approach

Slab Varies

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

Control Line (Schemes 5 & 6) -

Control Line (Projected from

Bridge) (Schemes 3 & 4) -

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required

Thrie-Beam

Guardrail-

⊊ Thrie-Beam

1'-2" x 10" x 1/8"

Thick Neoprene Pad See

Detail "A'

-Existing

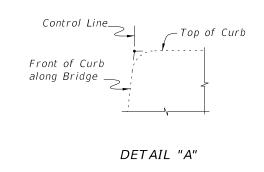
Approach

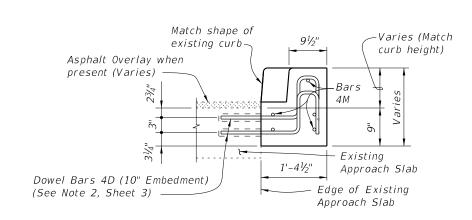
Slab

Varies 51/3" **

Guardrail-

Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

Varies **

 $(1'-0'' \pm Min.)$

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

Plumb (Max.) *

-Ç Guardrail Post:

and Recessed Nuts

Slope:

Embedment

<u>___Varie</u>s

_5" Min.

└ 3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

%" Ø Post Bolts (length varies)

-Guardrail Post Assembly "A", "B"

Existing Curb Overhang

 $\sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5)

or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}'')$ or $5\frac{1}{2}''$ Max.

drilled holes (5½" Max. Depth)

or "C" (See Roadway Plans)

Existing Wing Post

Existing Wing

(Type Varies)

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 01/01/08

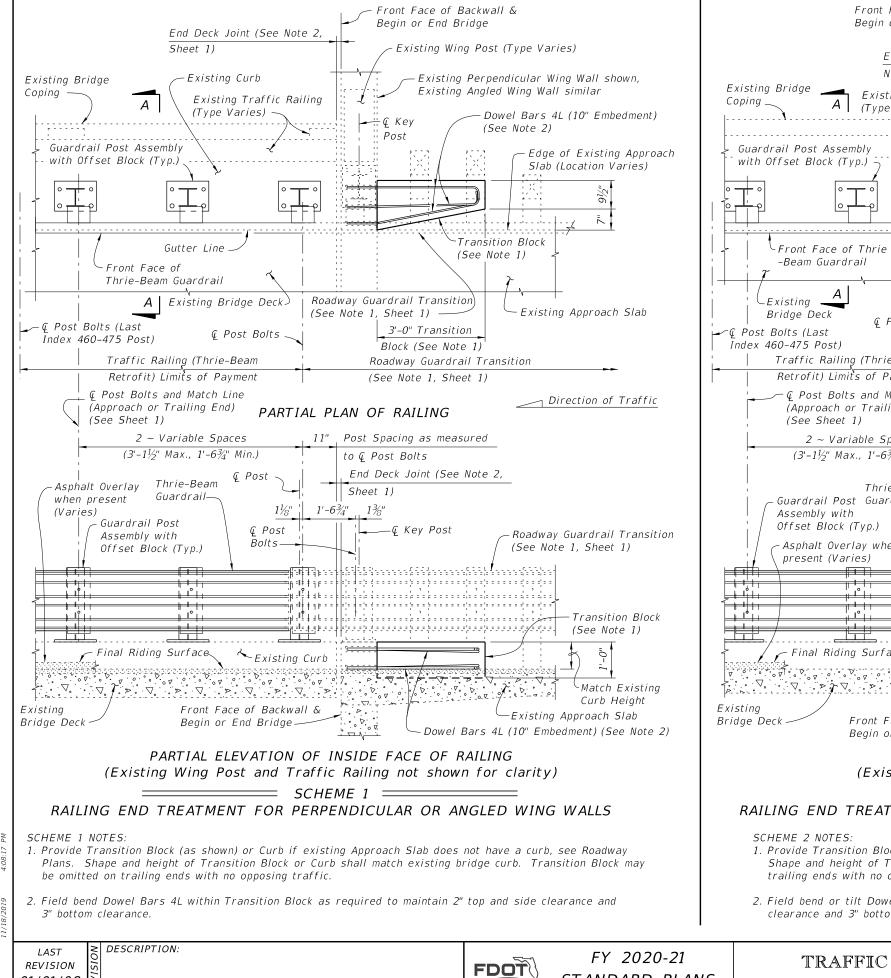
DESCRIPTION:

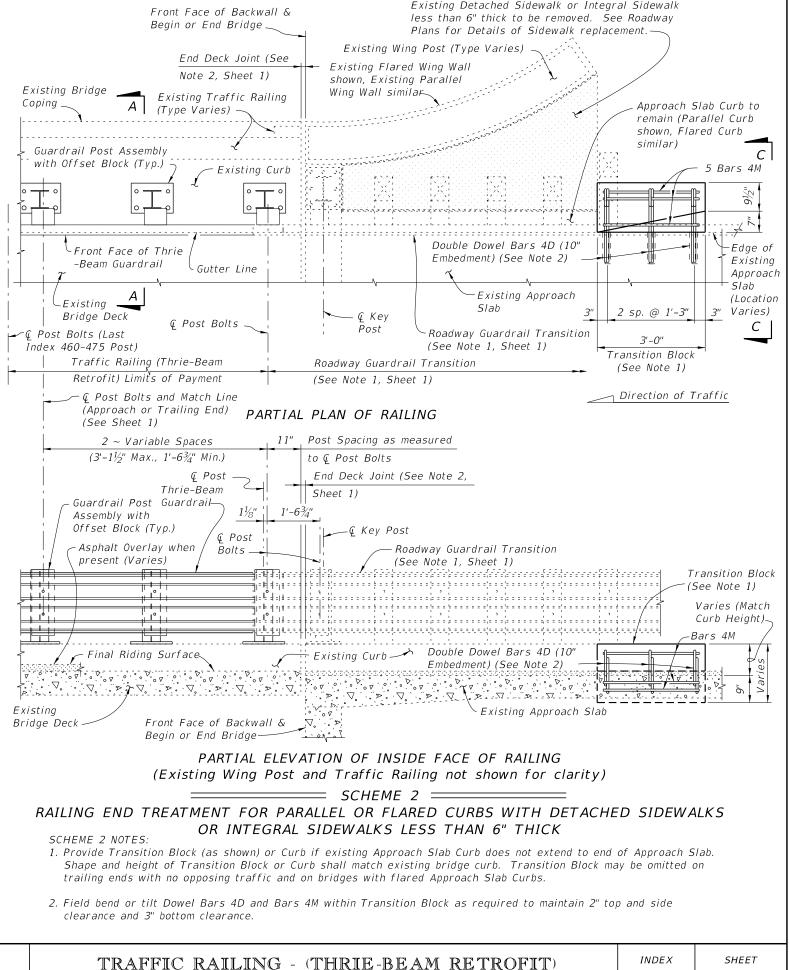
FDOT

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

INDEX

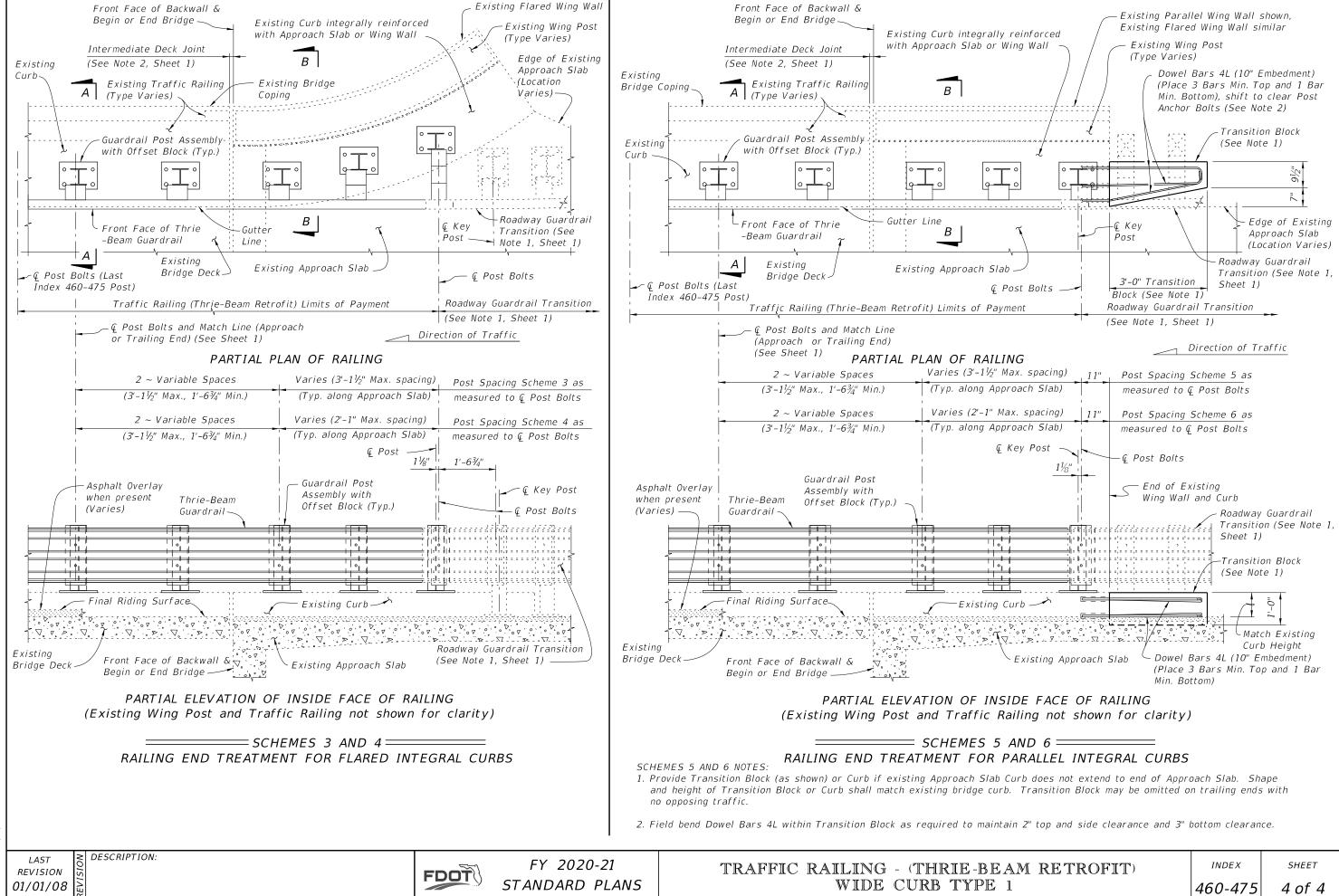
SHEET

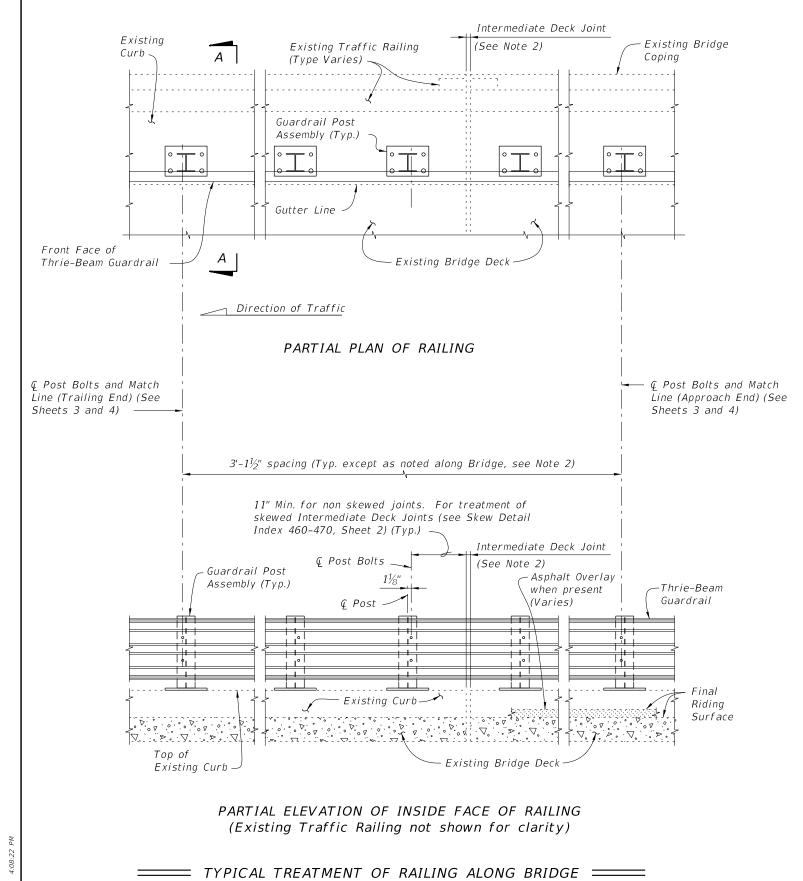




01/01/08

STANDARD PLANS





NOTES:

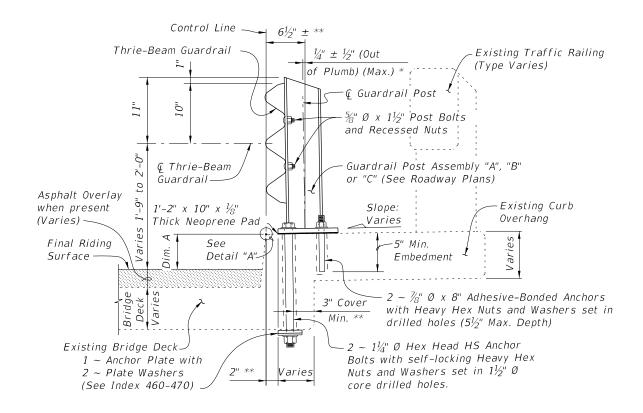
- 1. On approach end provide Index 536-002 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index 460-470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index 460-470.

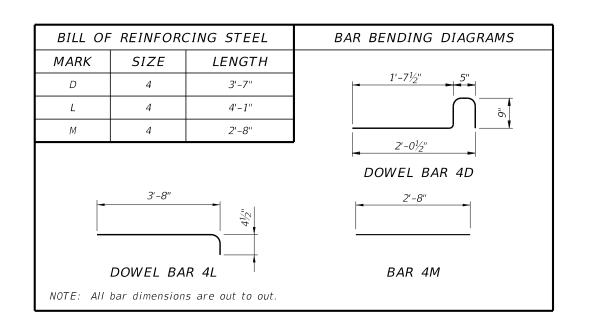
REVISION 01/01/08

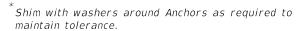
DESCRIPTION:

FDOT



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

Surface -

Approach

Slab Varies-

when present

(Varies) -

Thrie-Beam Guardrail along Bridge 🗲

Offset Block(s) as required

Thrie-Beam

Guardrail-

 ← Thrie-Beam

1'-2" x 10" x 1/3"

Thick Neoprene Pad

-Existing

Approach

Slab

Varies 51/2" **

Guardrail

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - Nominal Overhang

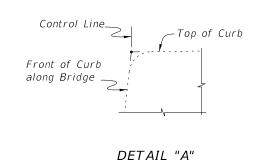
Control Line (Schemes 5 & 6)

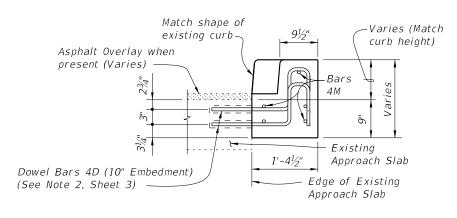
Control Line (Projected from

Bridge) (Schemes 3 & 4)

(Schemes 3 and 4 only)

Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

• © Guardrail

%" Ø x 8" Post

Slope:

Depth respectively).

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Bolts and Recessed Nuts

or "C" (See Roadway Plans)

Guardrail Post Assembly "A", "B"

Varies¦ Embedment

Existing Curb Overhang

__ 5" Min.

drilled holes (5½" Max. Depth)

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5) or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}'')$ or $5\frac{1}{2}''$ Max.

Plumb (Max.) *

Existing Wing Post

Existing Wing

Wall

%" Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

(Type Varies)

For location of Section C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 460-470, Sheet 3.

REVISION 07/01/08

DESCRIPTION:

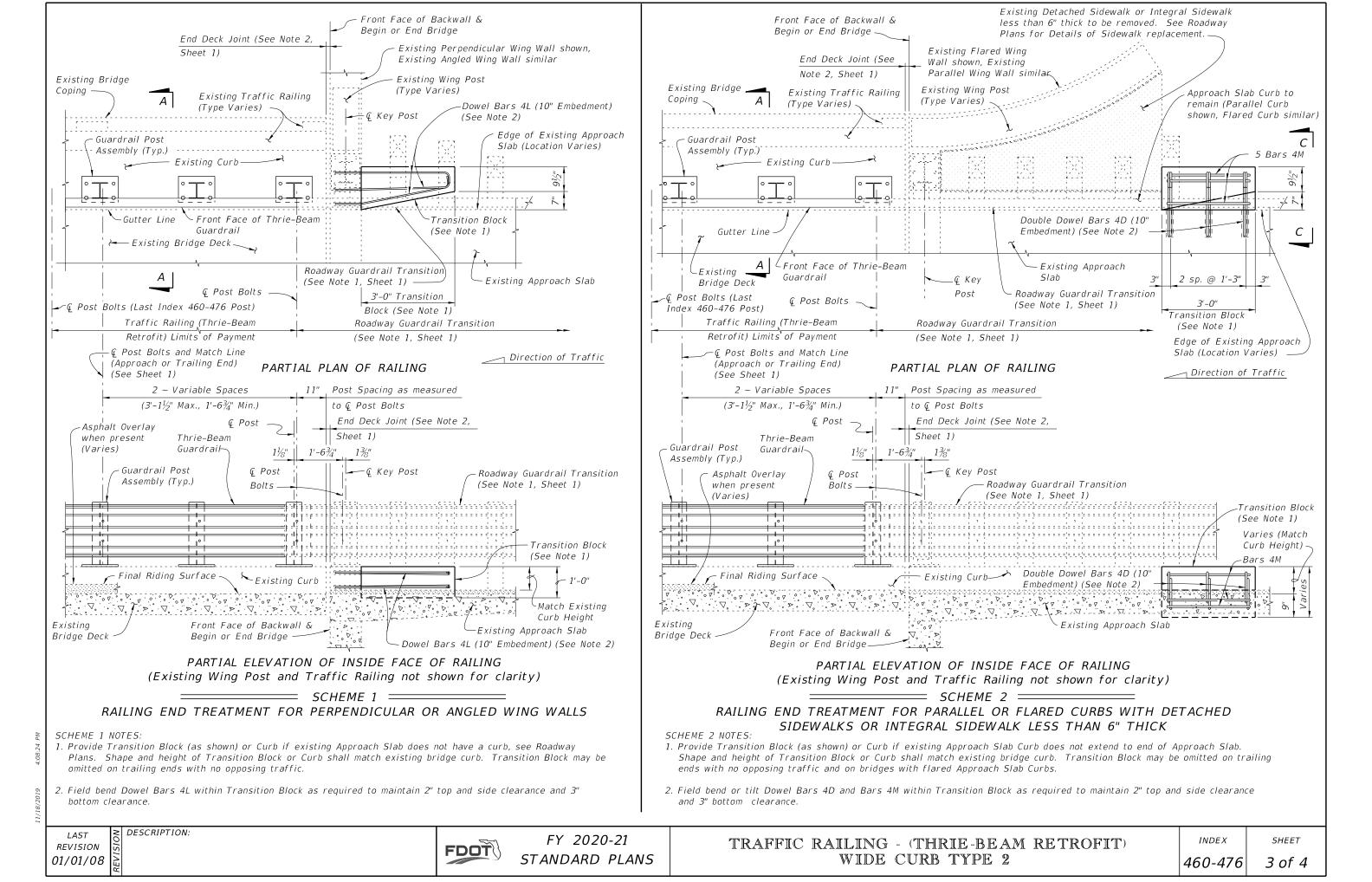
FDOT

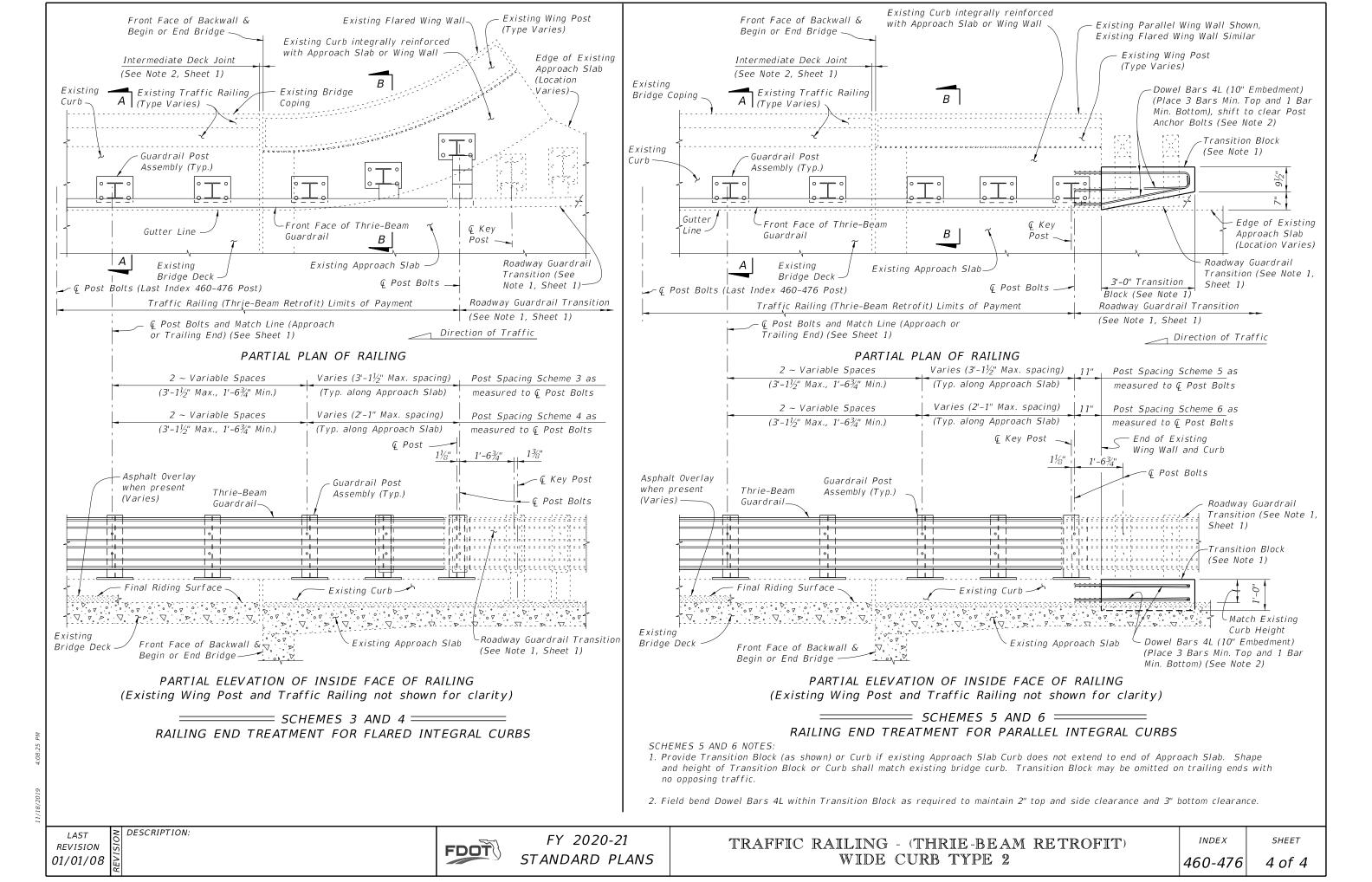
FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 2

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SHEET





CONCRETE: Concrete for Transition Blocks shall be Class II (Bridge Deck).

THRIE-BEAM PANEL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be $\frac{3}{4}$ " by $2\frac{1}{2}$ " slotted holes.

BOLTS, NUTS AND WASHERS: Bolts, nuts and round washers shall be in accordance with AASHTO M180. Plate Washers shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

COATINGS: All Nuts, Bolts, Anchors, and Washers shall be hot-dip galvanized in accordance with the Specifications.

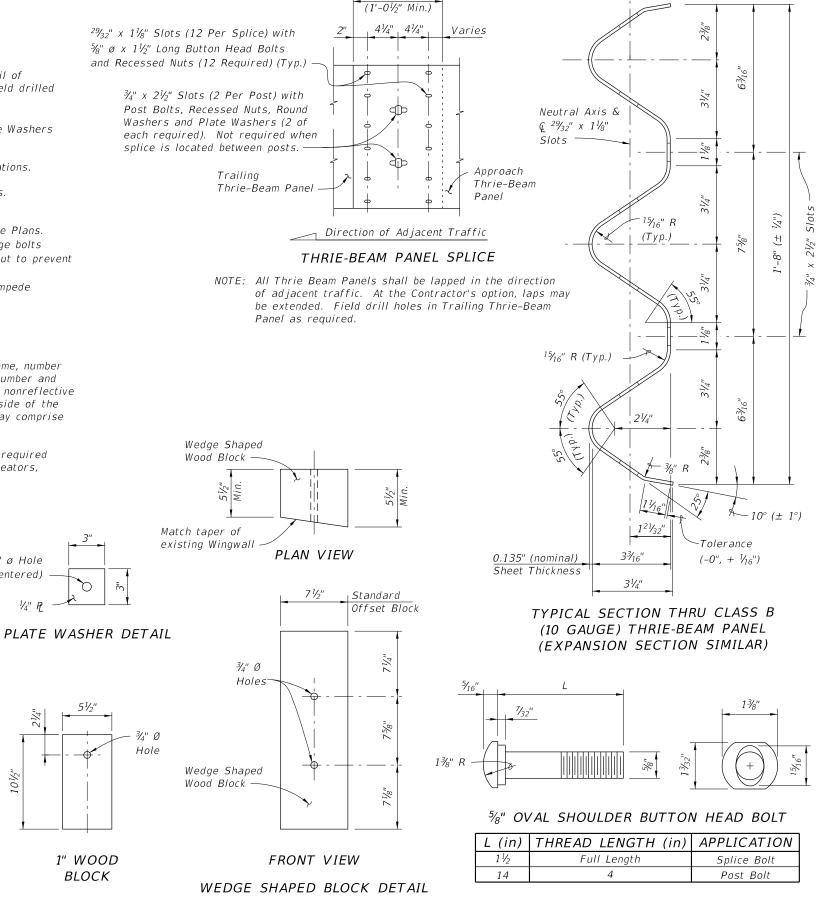
BRIDGES ON CURVED ALIGNMENTS: The details presented herein are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie-beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten bolts in 3¾" slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

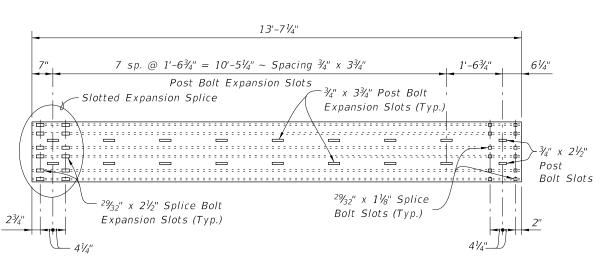
WOOD BLOCKS: All wood blocks, including required wedge shaped blocks shall be Pressure Treated Lumber in accordance with Specifications Section 955. Bolt holes in blocks to be centered ($\pm \frac{1}{4}$ ").

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie-Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent quardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

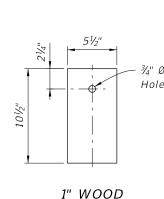
PAYMENT: Payment will be made under Thrie-Beam Panel Retrofit which shall include all materials and labor required to fabricate and install the retrofit railing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators, where required, will not be paid for directly but shall be considered incidental work.



Lap







¾" ø Hole

(centered)

DESCRIPTION:



FY 2020-21 STANDARD PLANS

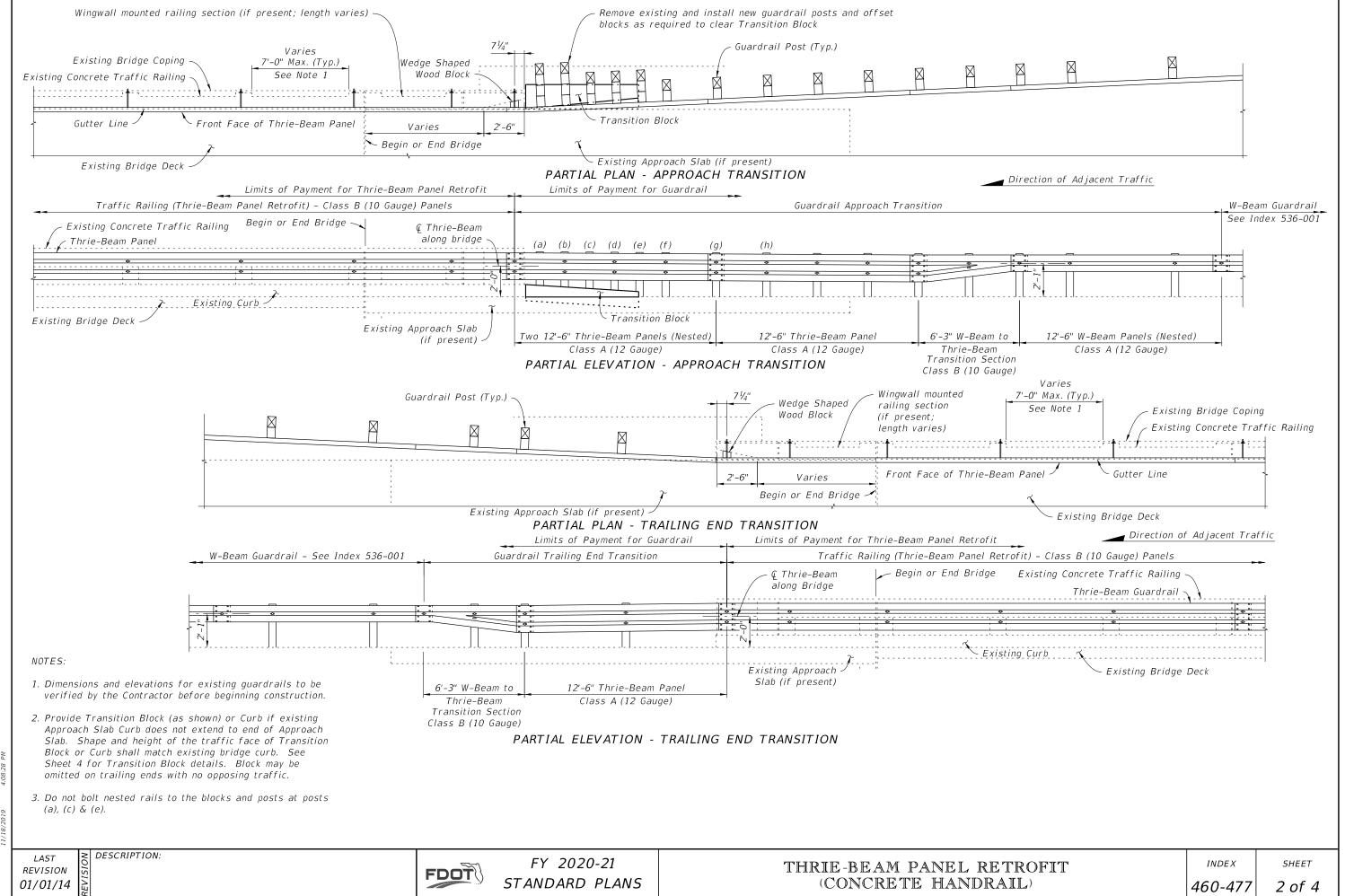
THRIE-BEAM PANEL RETROFIT (CONCRETE HANDRAIL)

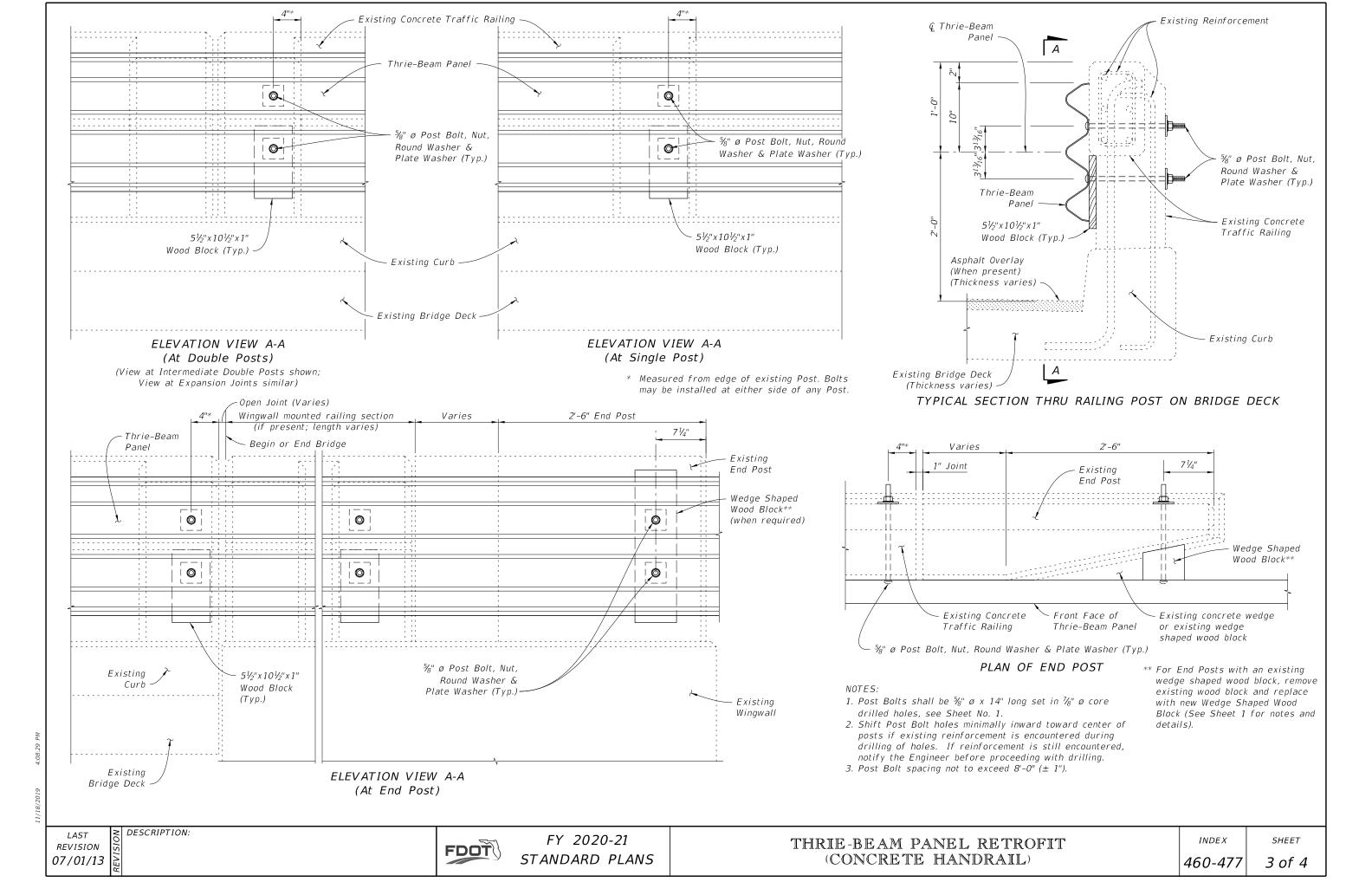
INDEX 460-477

SHEET 1 of 4

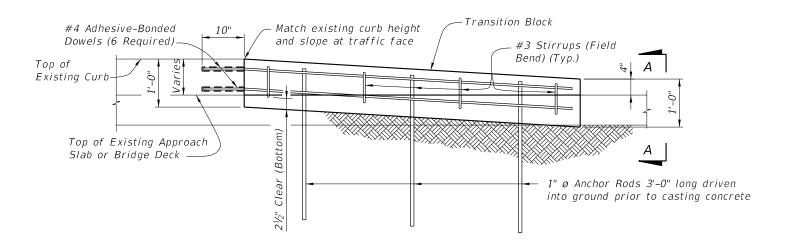
REVISION

07/01/14



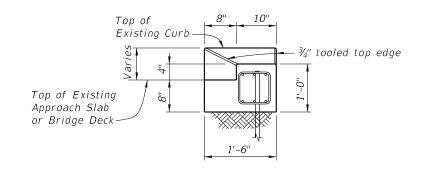


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

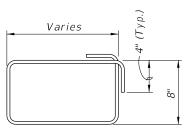


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK			
ITEM	UNIT	QUANTITY	
Concrete Class II (Bridge Deck)	CY	0.4	
Reinforcing Steel	LB	61	
Guardrail (Reset)	LF	12.5	



END VIEW A-A



#3 STIRRUP (FIELD BEND)

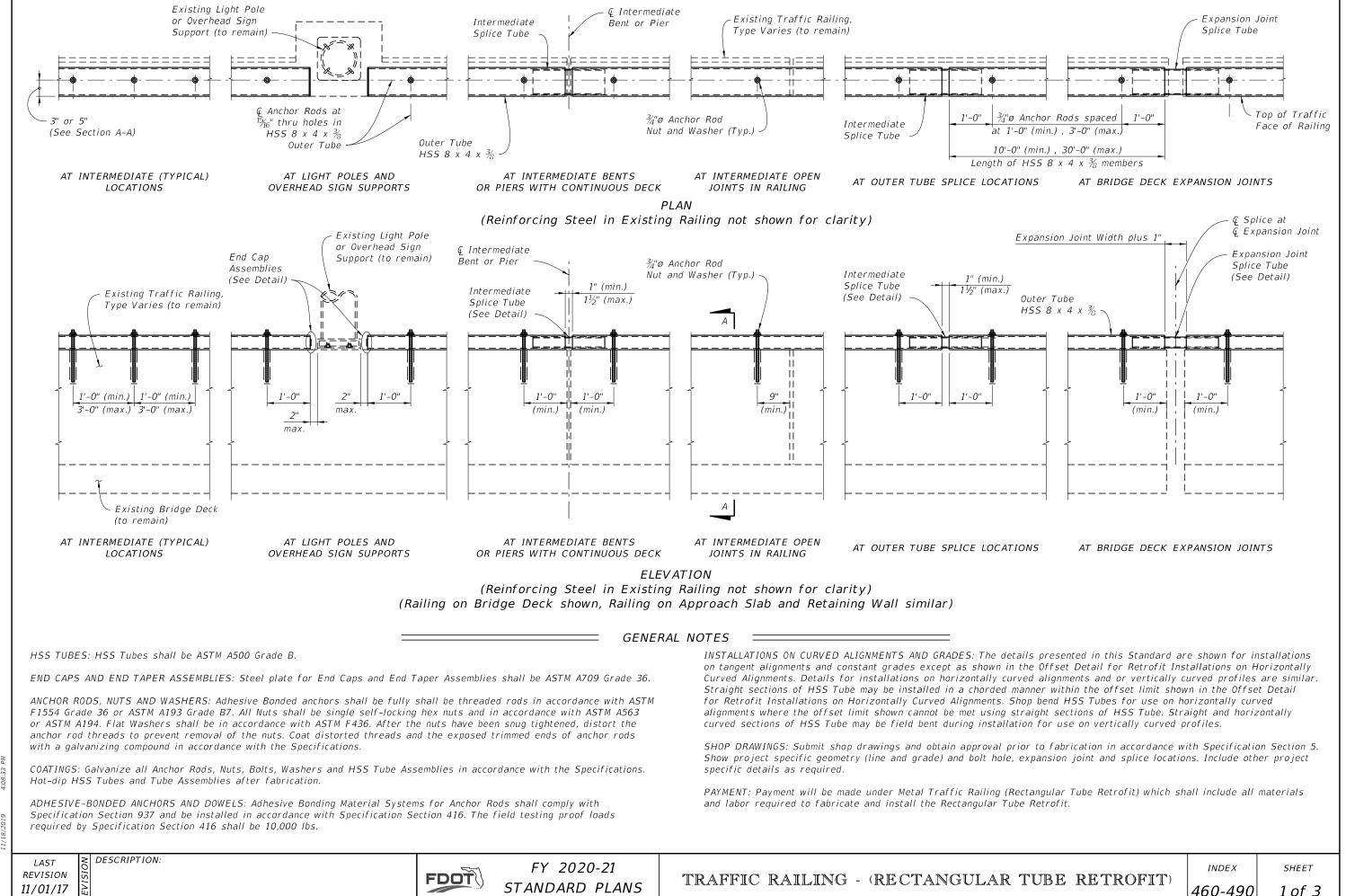
NOTES:

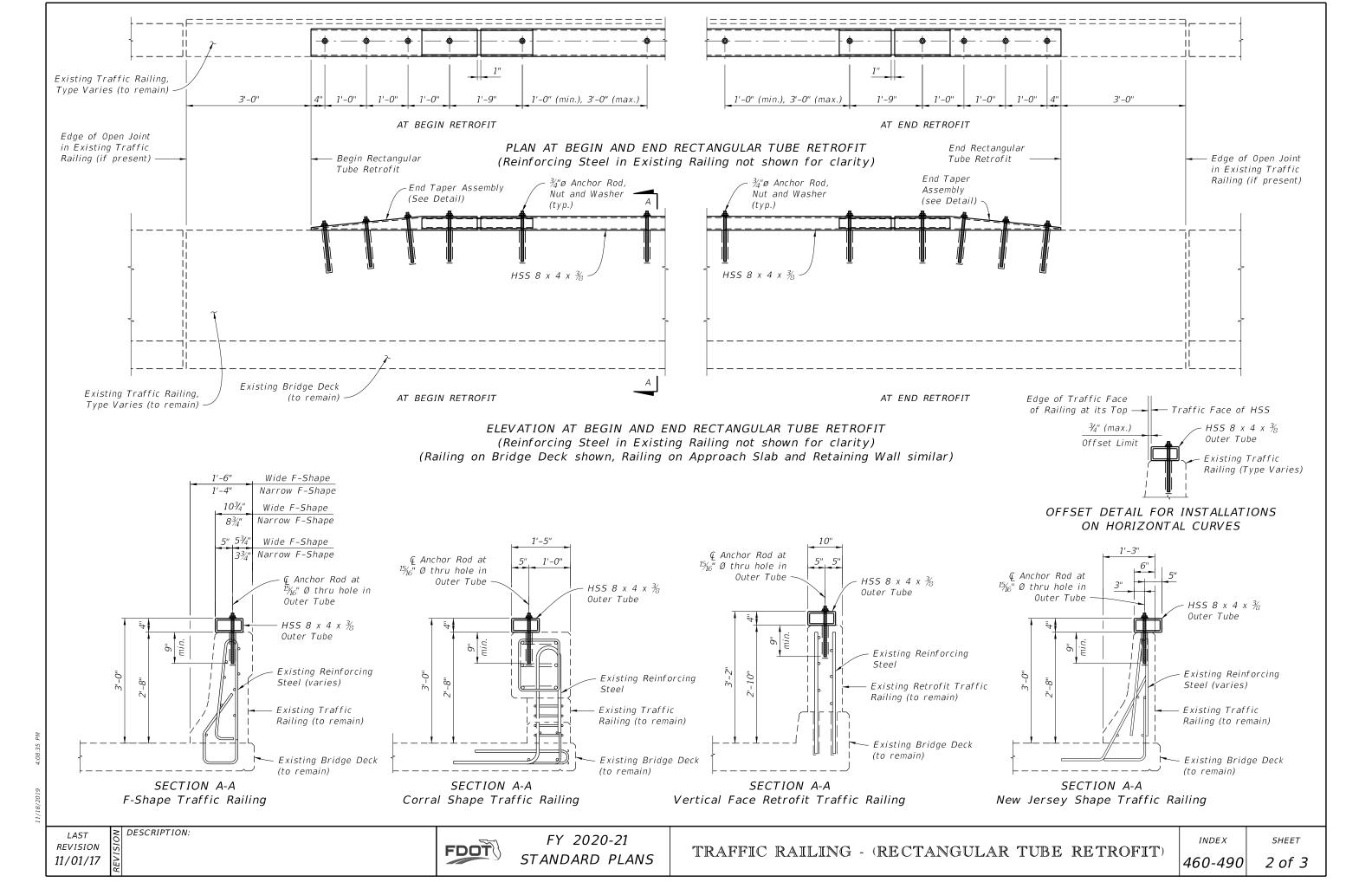
ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

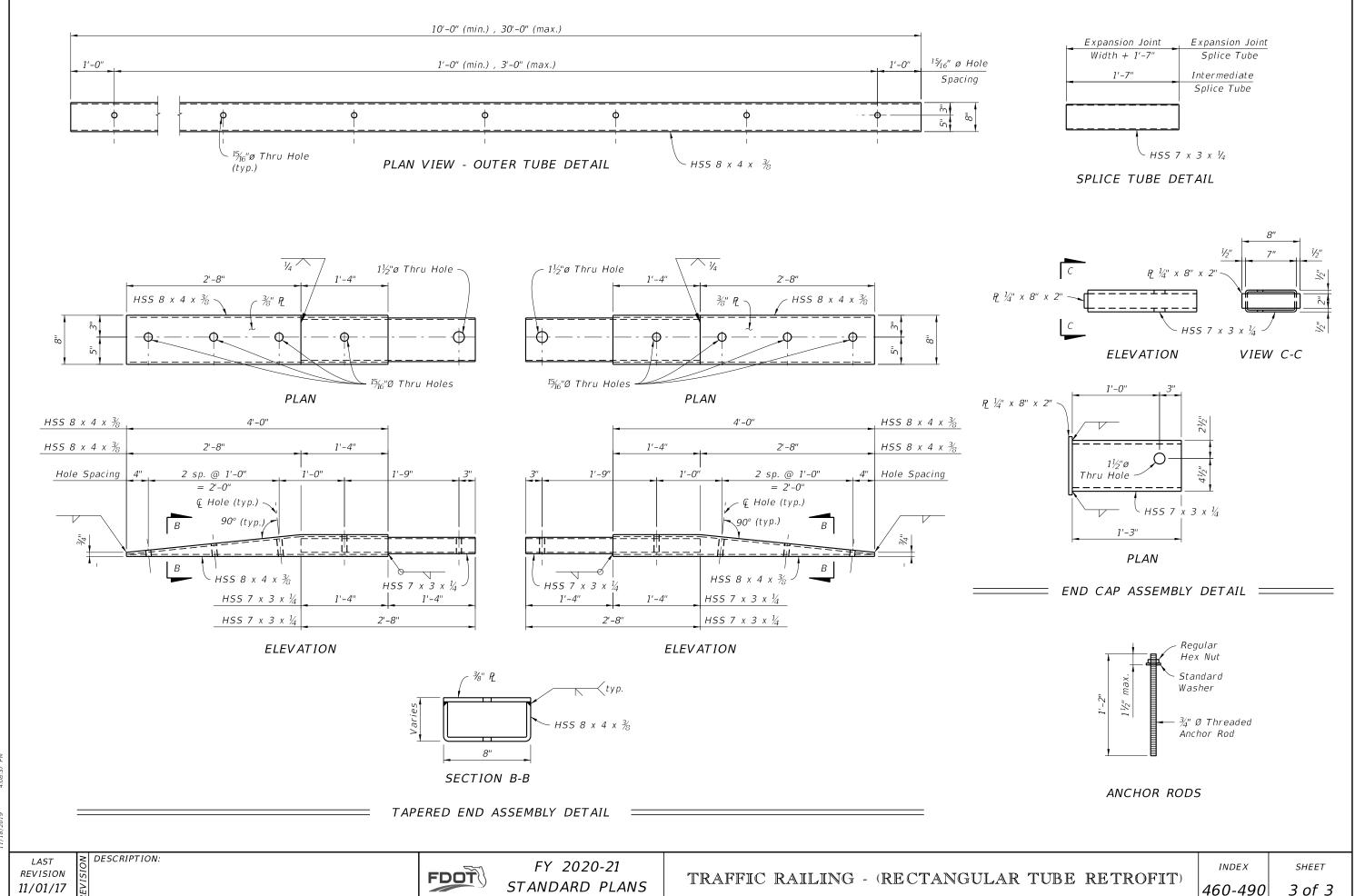
ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 (Type HV) and be installed in accordance with Specification Section 416.

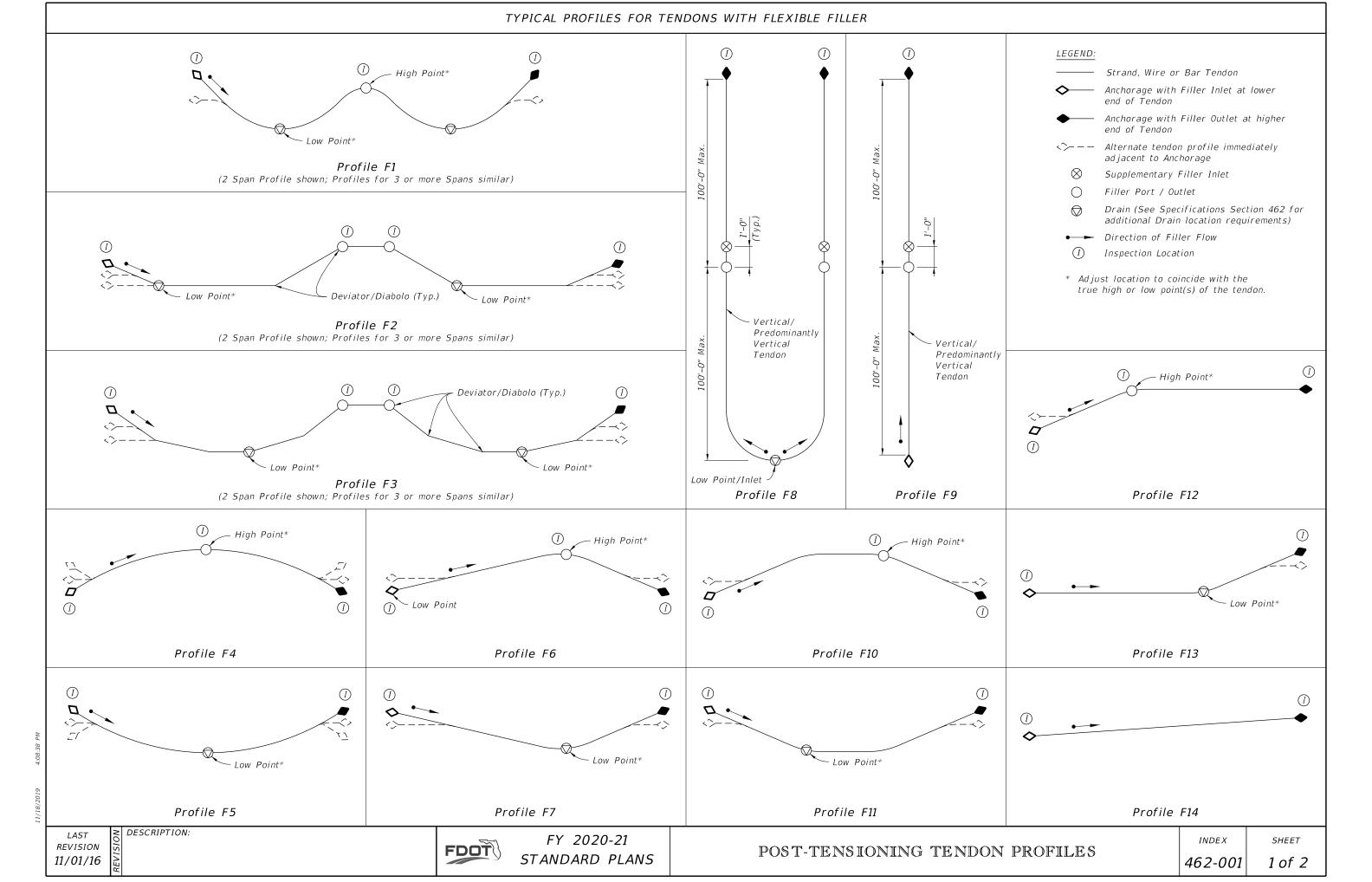
Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

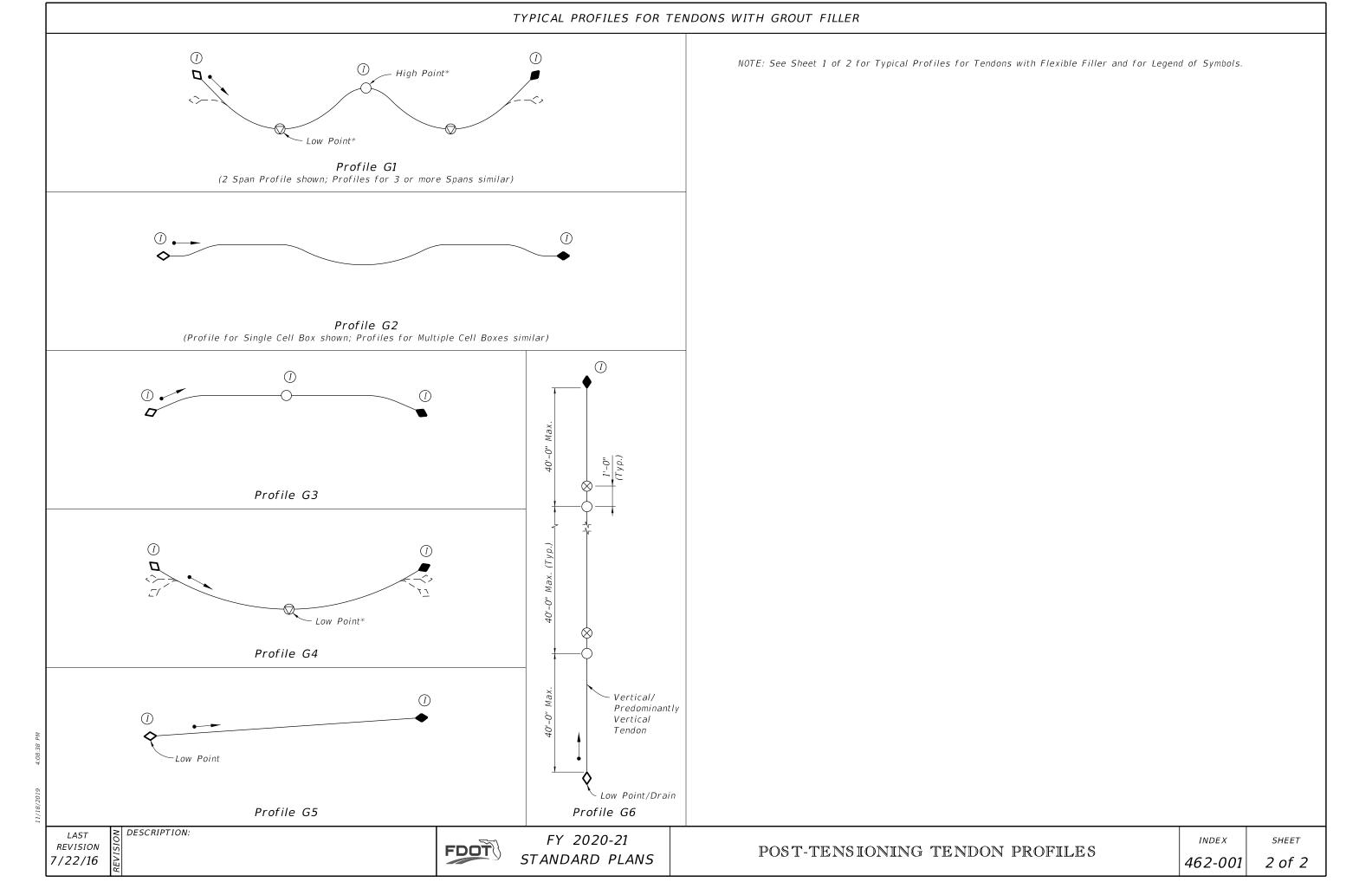
Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

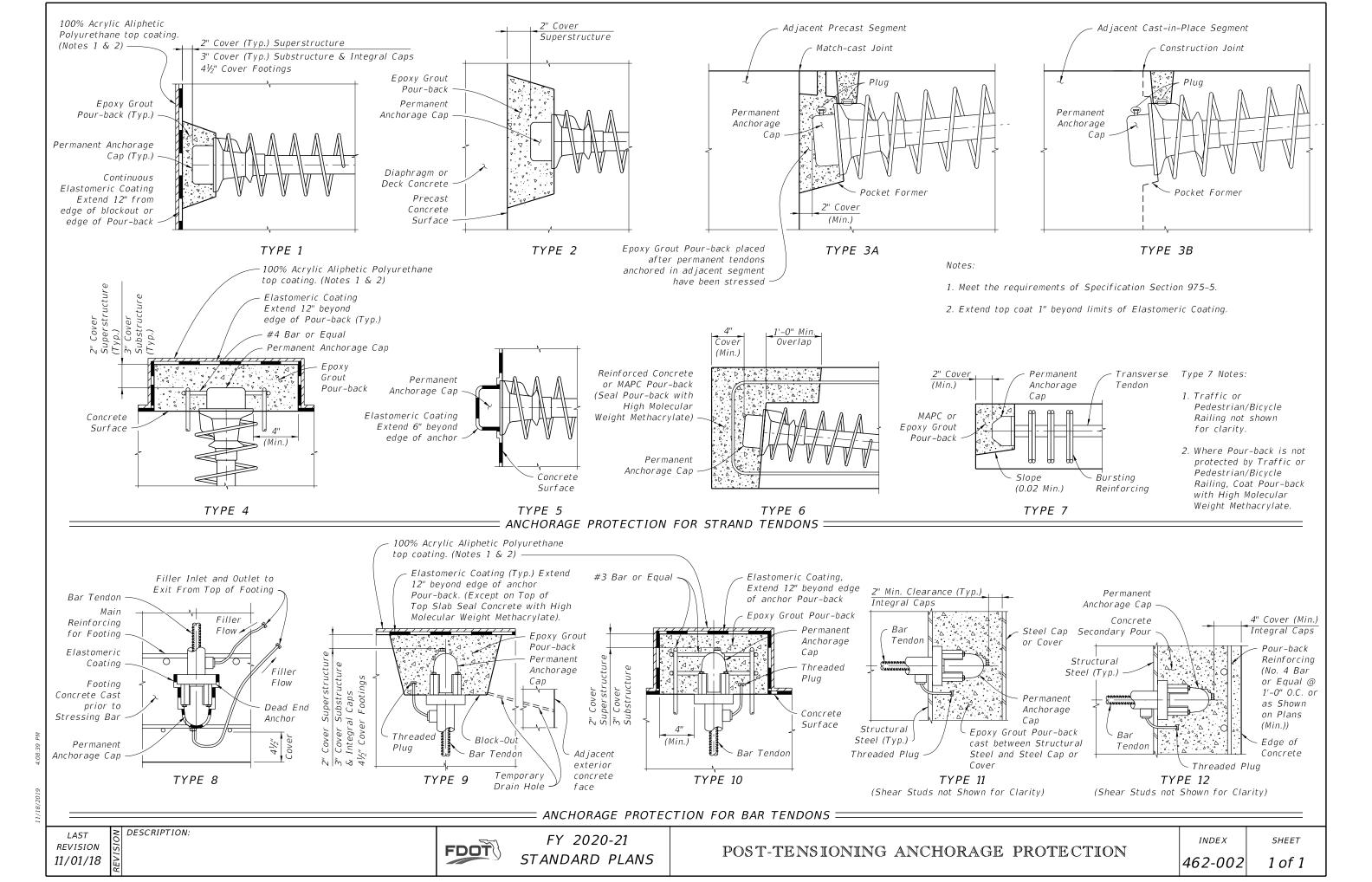


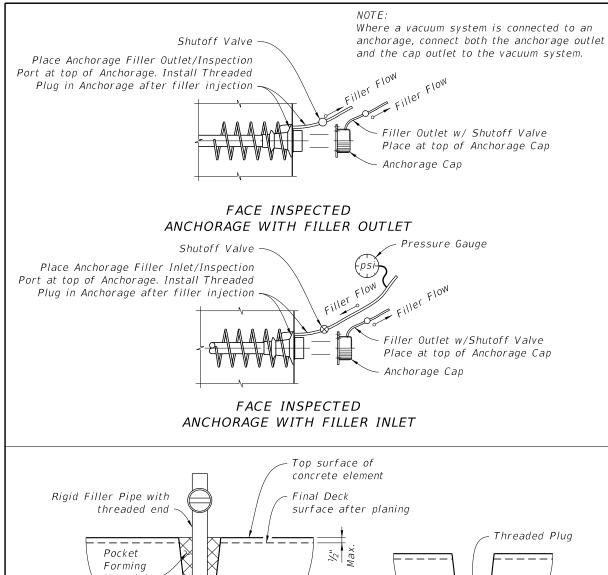


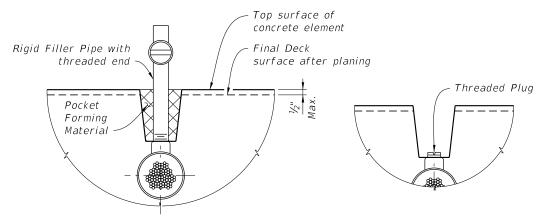


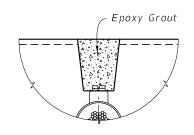












FILLER OUTLET CONNECTION TO DUCT

PROCEDURE:

1. After filler injection is completed, Remove Pocket Forming Material and Rigid Filler Pipe.

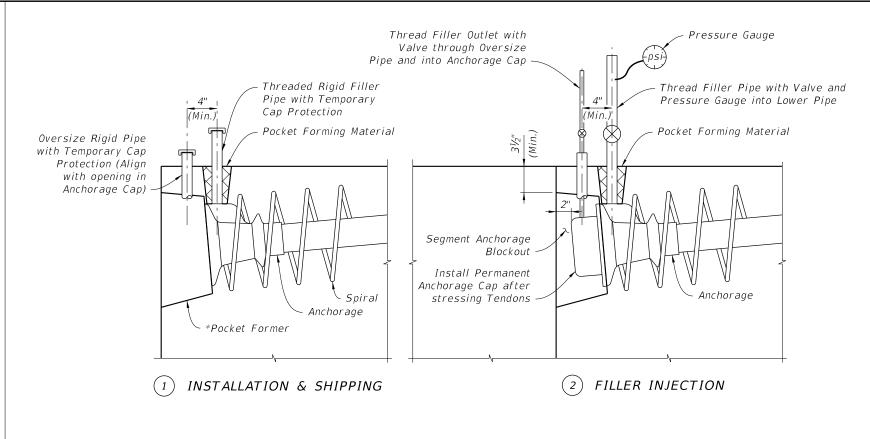
(2)POCKET

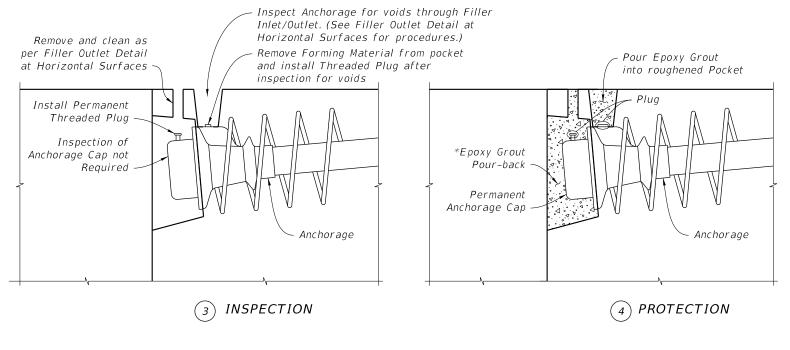
PREPARATION

- 2. Inspect Tendon for voids as necessary.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Clean threads and rethread as required.
- 5. Install Threaded Plug into Outlet to form a tight fit.
- 6. Clean and roughen sides of pocket.
- 7. Fill Pocket with Epoxy Grout.

(3) FILLING POCKET

= FILLER OUTLET DETAIL AT HORIZONTAL SURFACES =





NOTES:

- 1. Holes used for the Inspection and Filler Inlets/Outlets may be formed using tapered pipes or mandrels.
- 2. Where a vacuum system is connected to an anchorage, connect both the anchorage outlet and the cap outlet to the vacuum system.
- * Round () Pocket Former Gravity fed placement of epoxy grout acceptable Modified Square Pocket Former - Gravity fed placement of epoxy grout acceptable Square Pocket Former - Vacuum epoxy grouting required

REVISION 11/01/18

FDOT

FY 2020-21 STANDARD PLANS

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

TOP INSPECTED ANCHORAGE WITH FILLER

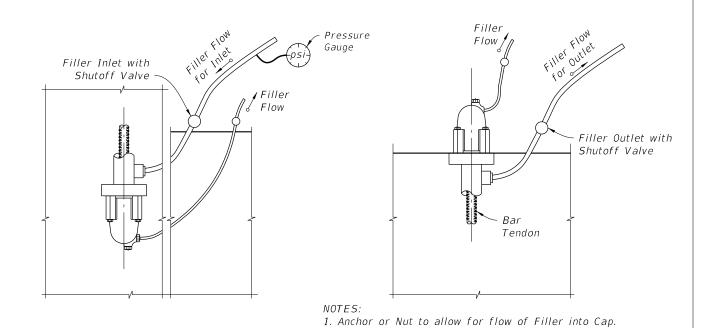
INLET INSTALLATION, FILLER INJECTION.

INSPECTION & PROTECTION

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SHEET

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INLET END

(EMBEDDED ANCHORAGE SHOWN; ANCHORAGE AT CONCRETE SURFACE SIMILAR)

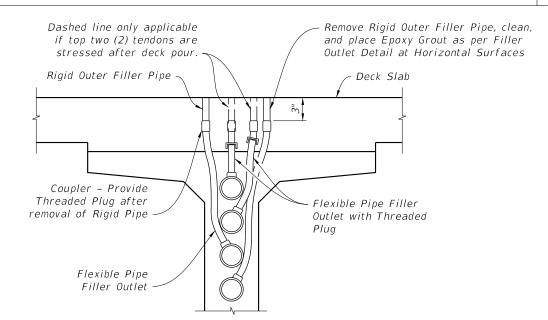
to the vacuum system. **OUTLET END**

2. Where a vacuum system is connected to an anchorage,

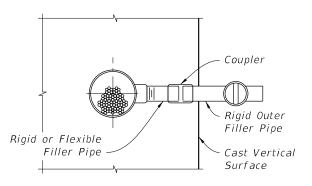
connect both the anchorage outlet and the cap outlet

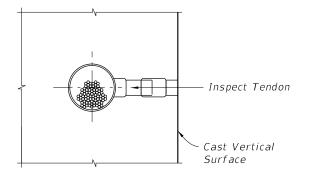
FILLER INLET AND OUTLET DETAILS FOR BAR TENDONS

(VERTICALLY ORIENTED TENDON SHOWN; HORIZONTALLY ORIENTED TENDON SIMILAR)



TENDONS AT HIGH POINTS AND 3' FROM HIGH POINTS (FILLER OUTLET)





(1) FILLER OUTLET CONNECTION TO TENDON

(3) FILLING POCKET

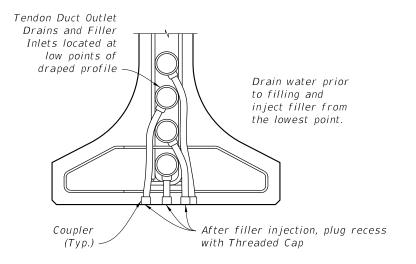
Epoxy Grout Threaded Plug Cast Vertical Surface

(2) POCKET PREPARATION

PROCEDURE:

- 1. Remove Rigid Filler Pipe or drill Grout in flexible pipe.
- 2. Inspect tendon for voids.
- 3. Vacuum inject as required. If grout is used, allow grout to cure. If flexible filler is used, replace filler displaced by inspection. Remove pipe used for vacuum injecting.
- 4. Install Threaded Plug into Outlet to form a tight fit.
- 5. Over-ream hole ($\frac{1}{4}$ " Ø over-ream). Clean and roughen sides.
- 6. Fill pocket with epoxy grout.

FILLER OUTLET DETAIL AT VERTICAL SURFACES



TENDONS AT LOW POINTS (FILLER INLET / DRAIN)

== FILLER INLET AND OUTLET DETAILS FOR I-GIRDERS ======

DETAILS FOR C.I.P. BOXES WITH INTERNAL TENDONS SIMILAR, WEB REINFORCING NOT SHOWN FOR CLARITY.

REVISION 11/01/16

DESCRIPTION:



FY 2020-21 STANDARD PLANS

POST-TENSIONING ANCHORAGE AND TENDON FILLING DETAILS

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462-003

2 of 2

- 14" SQUARE PRESTRESSED CONCRETE PILES Provide 14" Square Prestressed Concrete Piles of sufficient length to achieve a minimum embedment of 20' into soil having a blow count greater than or equal to 6 ($N \ge 6$). Pile splices and build-ups are not permitted. Use only 14" Square Prestressed Concrete Piles with 8 $\frac{1}{2}$ " diameter Low Relaxation Strands fabricated in accordance with Index 455–014.
- PLASTIC LUMBER AND STRUCTURAL COMPOSITE LUMBER WALES: Provide only Plastic Lumber (Thermoplastic Structural Shapes) and Structural Composite Lumber (Reinforced Thermoplastic Structural Shapes) Wales in accordance with Specification Section 973. Wales shall be continuous and spliced only at locations shown on the plans.
- PLASTIC LUMBER DECKING FOR CATWALKS: Provide Plastic Lumber decking for catwalks when called for in the Plans in accordance with Specification Section 973.

Install Plastic Lumber Decking according to manufacturer's recommendations using stainless steel $\#10 \times 3$ " (minimum) deck screws.

FIBERGLASS OPEN GRATING FOR CATWALKS: Provide Fiberglass Open Grating for catwalks when called for in the Plans. Fiberglass Open Grating shall be a heavy duty design suitable for exterior installations. Maximum gap opening on the walkway surface shall be $1\frac{1}{2}$ ". Design live loads and deflections shall be a 50 psf uniformly distributed load with a maximum deflection of $\frac{3}{8}$ " or L/120 at the center of a simple span and a concentrated load of 250 pounds with a maximum deflection of $\frac{1}{4}$ " at the center of a simple span. Color of Fiberglass Open Grating shall be gray or black.

Install Fiberglass Open Grating according to manufacturer's recommendations using stainless steel hardware, screws, bolts, nuts and washers. Attach Fiberglass Open Grating to Wales and Deck Supports at a 2'-0" maximum spacing so as to resist pedestrian live loads and uplift forces from wind, buoyancy and wave action.

- CLEARANCE GAUGE AND LIGHT: Clearance Gauge to be furnished and installed by the Contractor. Clearance Gauge width and numeral height is dependant on visibility distance. The required visibility distance shall be determined by the United States Coast Guard District Commander. Provide and install Clearance Gauge Light in accordance with Specification Section 510 and Index 510-001.
- NAVIGATION LIGHTS: Provide and install Navigation Lights in accordance with Specification Section 510, Index 510-001 and/or project specific details. Provide and maintain Temporary Navigation Lights during construction until permanent Navigation Lights are operational.
- BOLTS, THREADED BARS, NUTS, SCREWS AND WASHERS: Furnish stainless steel Bolts in accordance with ASTM F593 Type 316. Furnish stainless steel Threaded Bars in accordance with ASTM A193 Grade B8M. Furnish stainless steel Nuts in accordance with ASTM F594 Type 316. Furnish stainless steel Screws in accordance with ASTM F593 Type 305. Furnish stainless steel Washers compatible with Bolts, Threaded Rods and Nuts under heads and nuts. Torque Nuts on 1" diameter Bolts and Threaded Bars to 150 lb-ft. Keep threads on Bolts, Threaded Bars and Nuts free from dirt, coarse grime and sand to prevent galling and seizing during tightening.

SPLICE PLATES: Furnish Splice Plates in accordance with ASTM A240 Type 316.

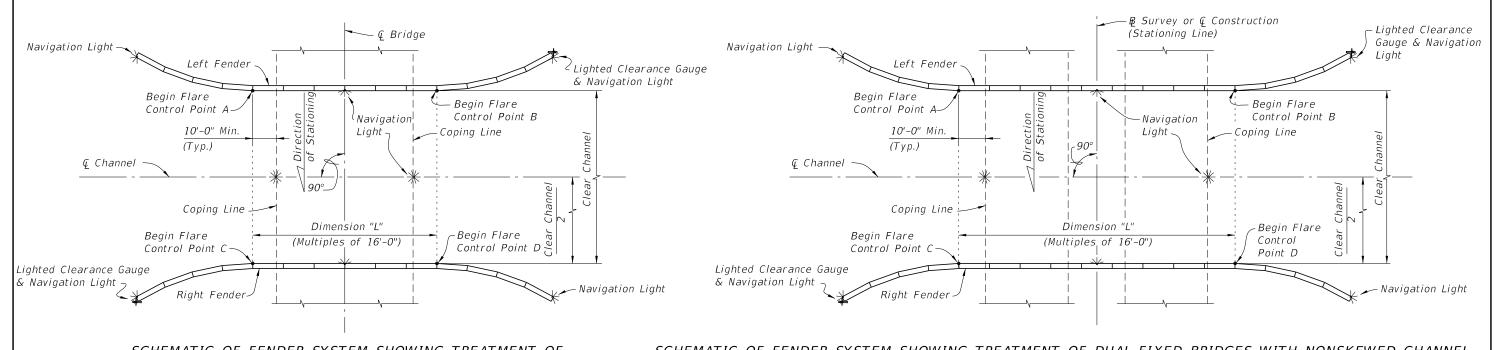
WIRE ROPE: Provide wire rope meeting one of the following requirements:

- 1. $\frac{1}{2}$ " diameter 6x19, 6x25 or 6x37 class IWRC Type 316 stainless steel wire rope with a minimum breaking strength of 18.000 lbs.
- 2. ½" diameter 6x19 galvanized wire rope with ultraviolet ray resistant polypropylene impregnation having an outside diameter of 5/8" with a minimum breaking strength of 22,000 lbs. Protect all ends with heat shrinkable end caps compatible with the rope's polypropylene that provide an effective water-tight seal.

FENDER SYSTEM ENERGY CAPACITY: Energy Capacity = 38 ft-k

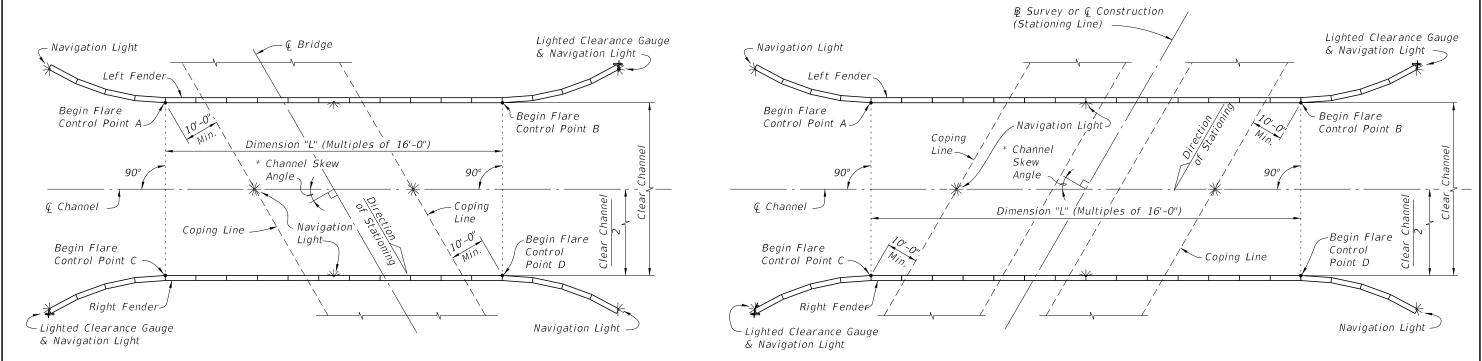
GENERAL NOTES

11/18/2019



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH NONSKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH NONSKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)



SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF SINGLE FIXED BRIDGE WITH SKEWED CHANNEL

SCHEMATIC OF FENDER SYSTEM SHOWING TREATMENT OF DUAL FIXED BRIDGES WITH SKEWED CHANNEL (PARALLEL DUAL FIXED BRIDGES SHOWN, NONPARALLEL DUAL FIXED BRIDGES SIMILAR)

* See Structures Plans, Plan and Elevation and Foundation Layout Sheets for magnitude and orientation of Channel Skew Angle.

CROSS REFERENCES:

For Stations and Offsets of referenced Control Points A, B, C and D, Dimension "L" and Clear Channel Width see Fender System Table of Variables in Structures Plans.

For Navigation Light Details see Design Standards Index 510-001.

LAYOUT GEOMETRY

REVISION 07/01/11

DESCRIPTION:

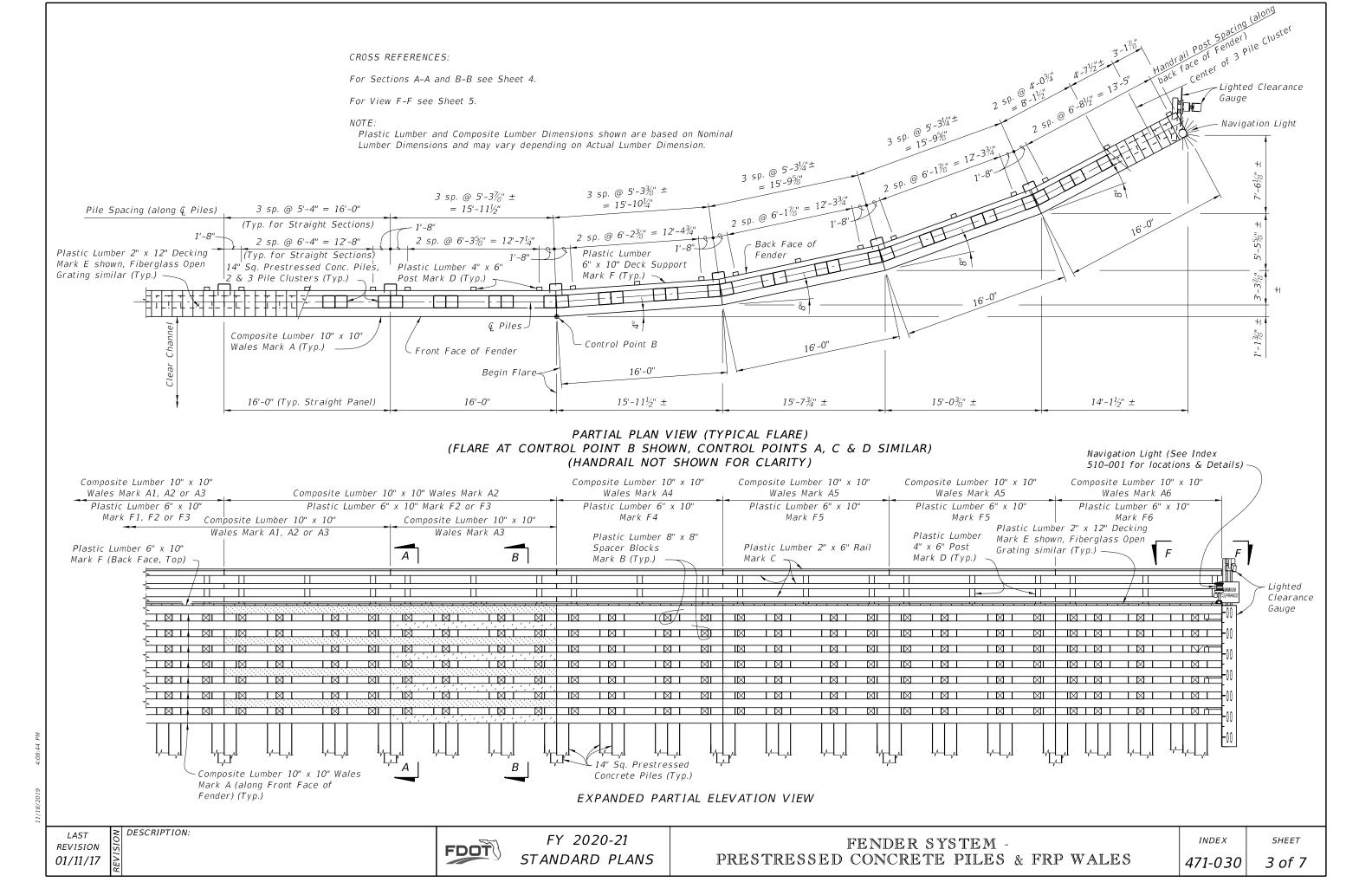
FDOT

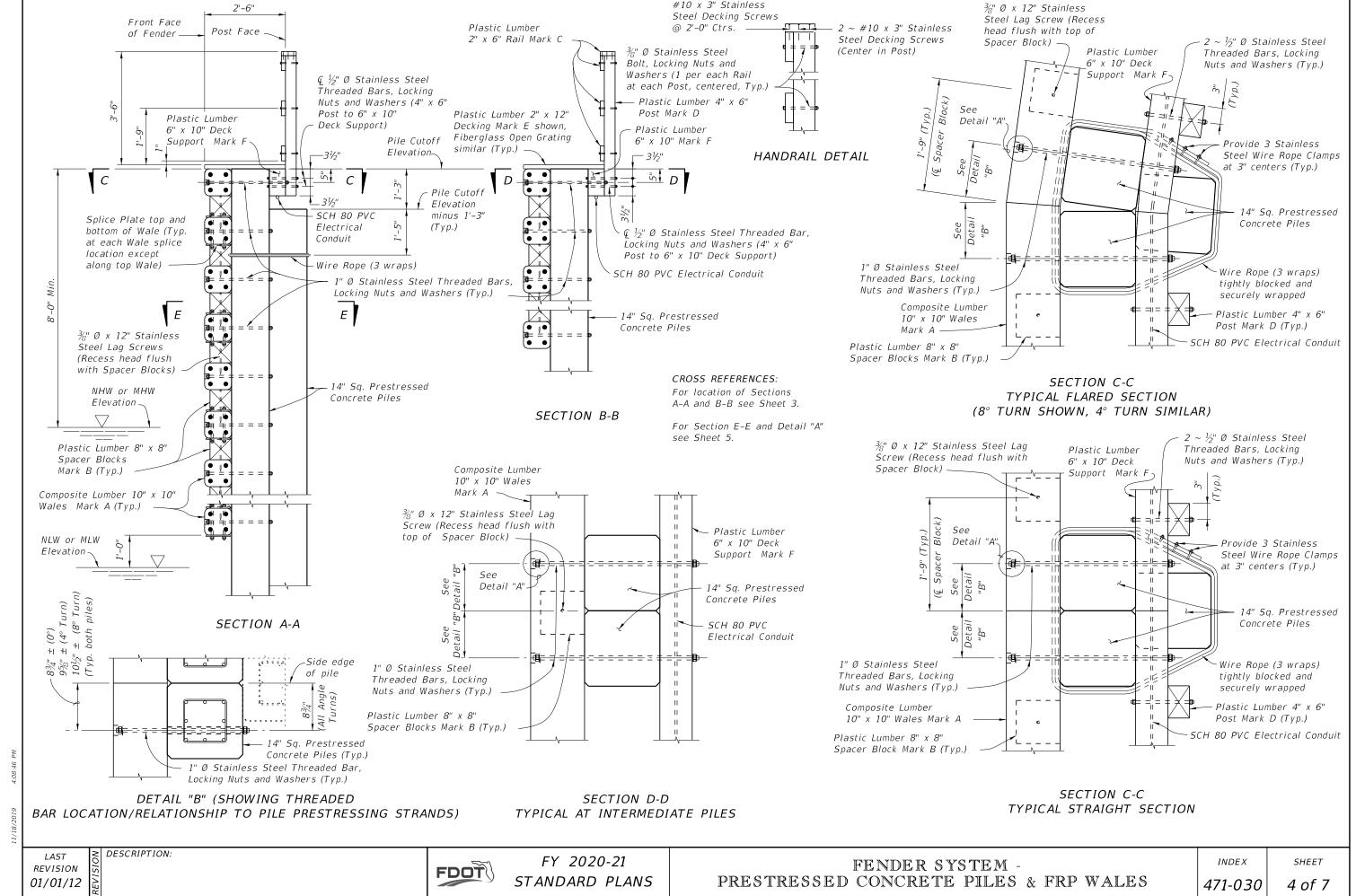
FY 2020-21 STANDARD PLANS

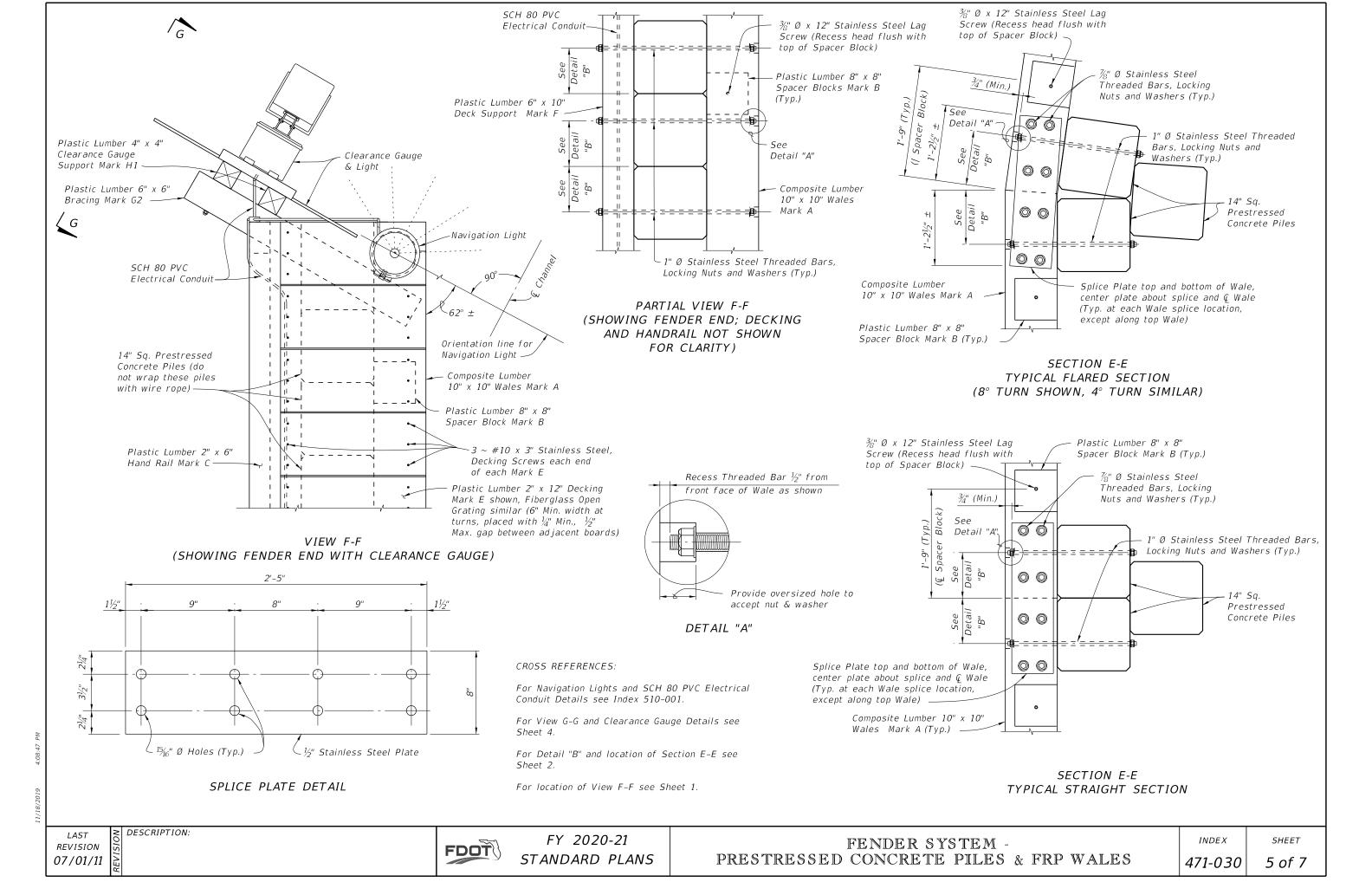
PRESTRESSED CONCRETE PILES & FRP WALES

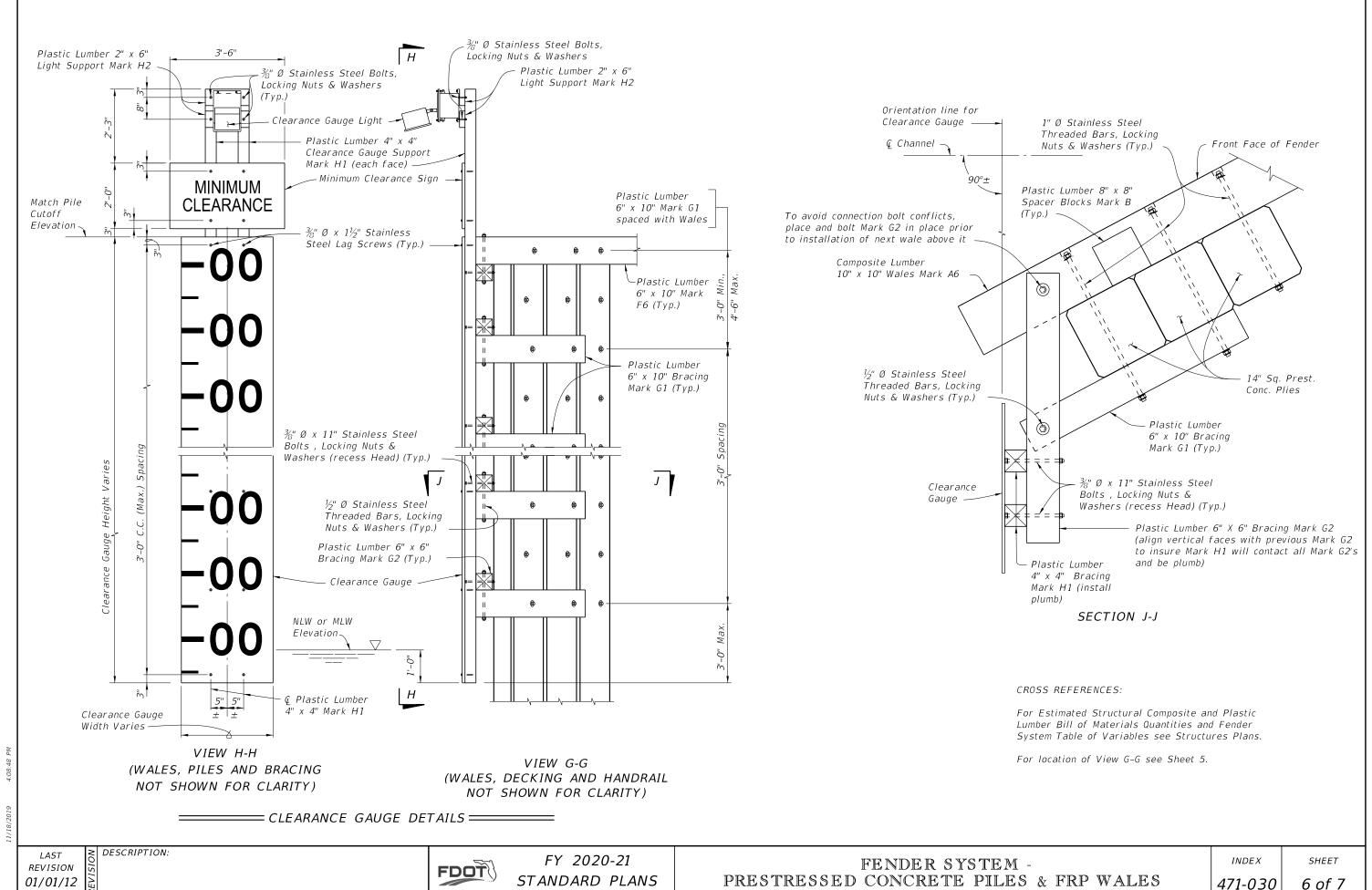
INDEX 471-030

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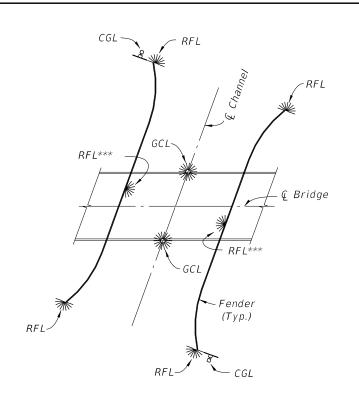


* STRUCTURAL COMPOSITE LUMBER BILL OF MATERIALS					
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY
A1	10" X 10" COMPOSITE LUMBER	32'-0" (STRAIGHT)	266.6	nber	
A2	10" X 10" COMPOSITE LUMBER	32'-0"	266.6	and Plastic Lumber	res Plans
АЗ	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	mposite and	in Stru
A4	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	tructural Co	Materials Table
A5	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	ate	Bill of Ma
A6	10" X 10" COMPOSITE LUMBER	16'-0"	133.3	See	

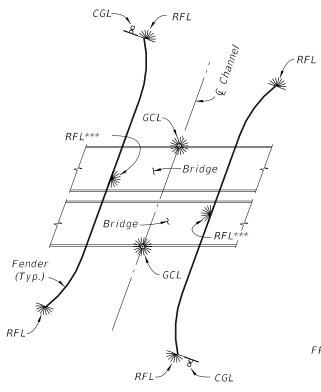
* PLASTIC LUMBER BILL OF MATERIALS						
MARK	SIZE (NOMINAL)	DIMENSIONS	BOARD FT. PER EACH	NO. REQD.	QUANTITY	
В	8" X 8" PLASTIC LUMBER	8" (STRAIGHT)	3.6			
С	2" X 6" PLASTIC LUMBER	16'-0" (STRAIGHT) (Trim & Miter Ends as required)	16.0			
D	4" X 6" PLASTIC LUMBER	4'-4" (STRAIGHT)	8.7			
** E	2" X 12" PLASTIC LUMBER	2'-6" (STRAIGHT) (Miter as required, 6" Min. width)	5.0			
F 1	6" X 10" PLASTIC LUMBER	32'-0" (STRAIGHT)	160.0	mber		
F2	6" X 10" PLASTIC LUMBER	31'-11"	159.6	Estimated Structural Composite and Plastic Lumber	ıres Plans	
F3	6" X 10" PLASTIC LUMBER	15'-11"	79.6	omposite an	e in Structu	
F4	6" X 10" PLASTIC LUMBER	15'-91/4"	78.8	structural Co	Bill of Materials Table in Structures Plans	
F5	6" X 10" PLASTIC LUMBER	15'-81/4"	78.4		Bill of Ma	
F6	6" X 10" PLASTIC LUMBER	15'-101/4"	79.3	See		
G 1	6" X 10" PLASTIC LUMBER	3'-8" (STRAIGHT)	18.3			
G2	6" X 6" PLASTIC LUMBER	4'-1" (STRAIGHT)	12.3			
H1	4" X 4" PLASTIC LUMBER	PILE CUTOFF ELEV. MINUS NLW OR MLW ELEV. PLUS 5'-6" (STRAIGHT)	1.3 PER LF EACH			
Н2	2" X 6" PLASTIC LUMBER	1'-2" (STRAIGHT)	1.2			

^{*} All Plastic Lumber and Composite Lumber Dimensions and Quantities shown are based on Nominal Lumber Dimensions and may vary depending on Actual Lumber Dimension.

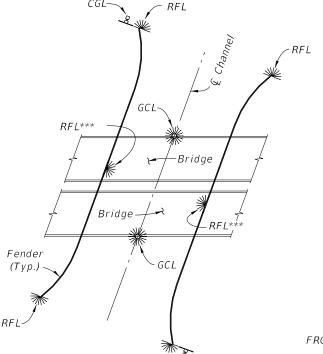
^{**} Provide Fiberglass Open Grating in lieu of 2" X 12" Plastic Lumber when called for in the Plans. Mounting hardware shall be Stainless Steel, install per Manufacturer's recommendations. See Structures Plans for Notes and Details.



NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITH FENDERS



NAVIGATION LIGHT SYSTEM SCHEMATIC



FOR DUAL BRIDGES WITH FENDERS

RFL or RCL * -RFL or RCL * RFL or RCL * RFL or RCL * GCL-^LRFL ** Channel Edge (Typ.) ⊀RFL ** Bridge - RFL ** GCL~RFL or RCL RFL or RCL * -RFL or RCL *

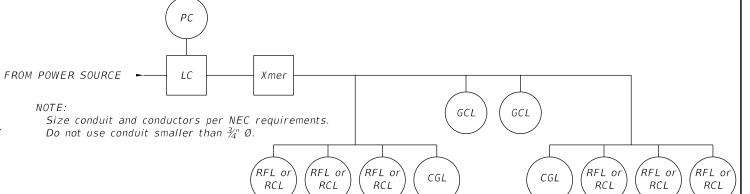
NAVIGATION LIGHT SYSTEM SCHEMATIC FOR SINGLE BRIDGE WITHOUT FENDERS

NAVIGATION LIGHT SYSTEM SCHEMATIC FOR DUAL BRIDGES WITHOUT FENDERS

- * Use RFL when Pier is at Channel Edge and see CFR, Title 33, part 118 for Mounting Height restrictions. Use RCL otherwise.
- ** Mounted only on the Pier that defines CM, otherwise does not apply.
- *** RFL to be located at mid length of straight portion of fender.

NAVIGATION LIGHT NOTES:

1. Provide Navigation Light System in compliance with Specifications Section 510.



TYPICAL ELECTRICAL SCHEMATIC DIAGRAM

POWER CONDUCTORS					
DISTANCE	VOLTS	CONDUCTOR	TRANSFORMER		
(feet)					
0 - 75	120	#12 AWG	N/A		
75 - 500	120 or 240	#10 AWG	N/A		
500-1000	240	#10 AWG	N/A		
1000-2000	480	#10 AWG	2 KVA		
2000-5000	480	#8 AWG	2 KVA		
5000-10000	480	#6 AWG	2 KVA		
over 10000	480	#4 AWG	2 KVA		

LEGEND

SYMBOL DESCRIPTION

Lighting Contactor

Photocell Control

Xmer Transformer (If Required)

 $\lceil RFL \rceil$ Red Pier/Fender Light (180° visibility) or RCL Red Channel Margin Light (180° visibility)

Green Center Channel Light (360° visibility)

△ CGL Clearance Gauge Light

> Channel Margin or Pier inner surface whichever defines Channel Edge.

REVISION 11/01/17

DESCRIPTION:

RFL or RCL *

FDOT

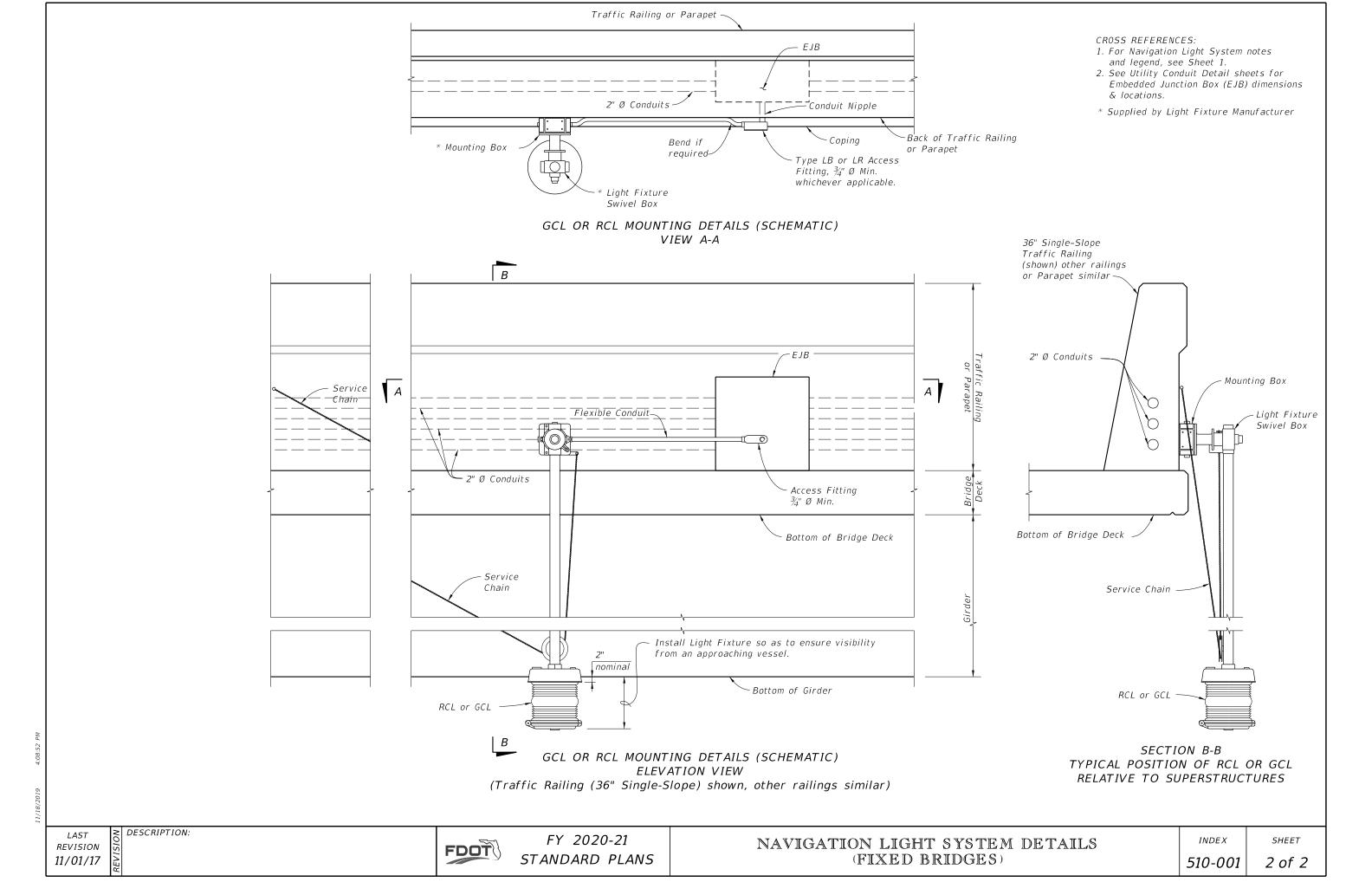
FY 2020-21 STANDARD PLANS

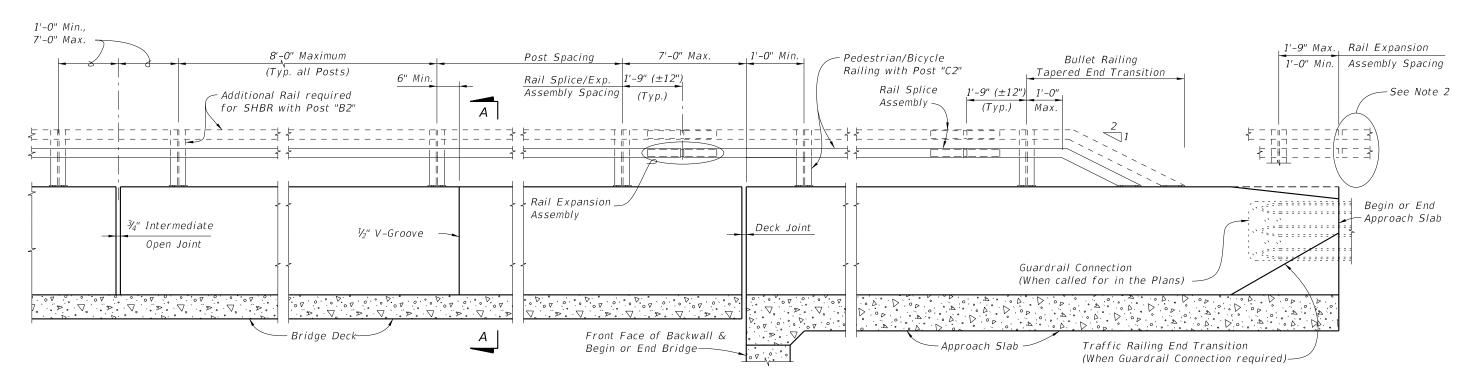
(FIXED BRIDGES)

INDEX 510-001

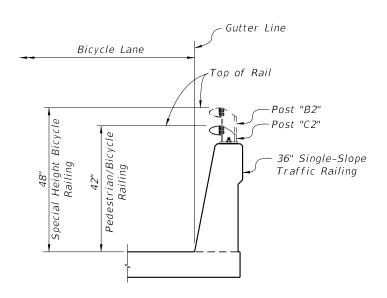
SHEET 1 of 2

NAVIGATION LIGHT SYSTEM DETAILS





ELEVATION OF INSIDE FACE OF TRAFFIC RAILING WITH PEDESTRIAN/BICYCLE BULLET RAILING



SECTION A-A TYPICAL SECTION THRU BRIDGE DECK (APPROACH SLAB SIMILAR)

NOTES:

- 1. A Bullet Railing Tapered-End Transition is required for all approach ends of Bullet Railings on Traffic Railings. When Guardrail Connection is required teminate the Bullet Railing Tapered-End Transition at begining of the Traffic Railing End Transition.
- 2. Where Bullet Railing continues on retaining wall mounted Traffic Railings or Barriers, provide a Bullet Railing Tapered End Transition at the terminus of the Bullet Railing.

CROSS REFERENCES:

Work in conjunction with Index 515-022.

For Traffic Railing Details, Reinforcement and Notes see Index 521-427.

REVISION 11/01/17

DESCRIPTION:

FDOT

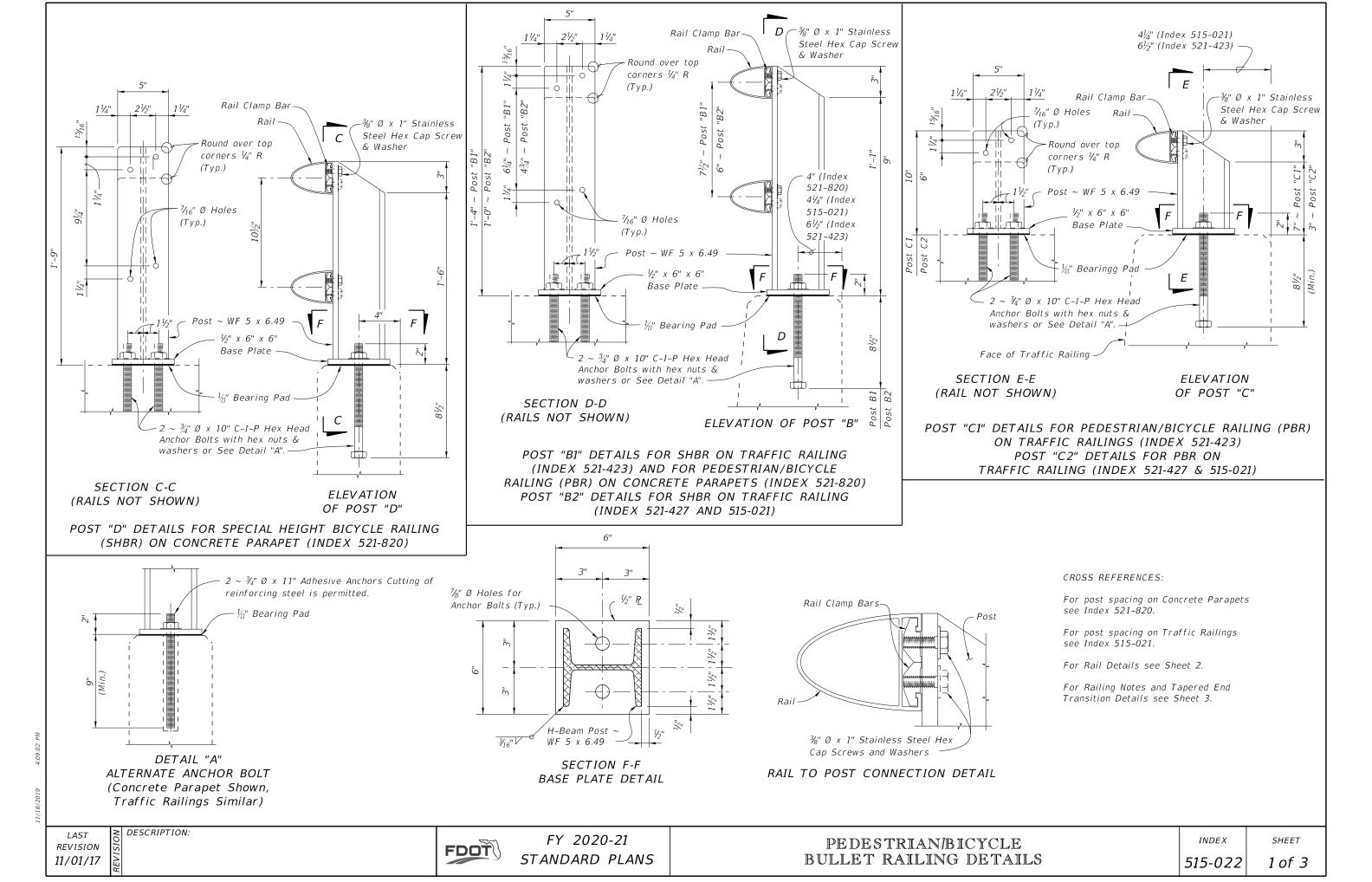
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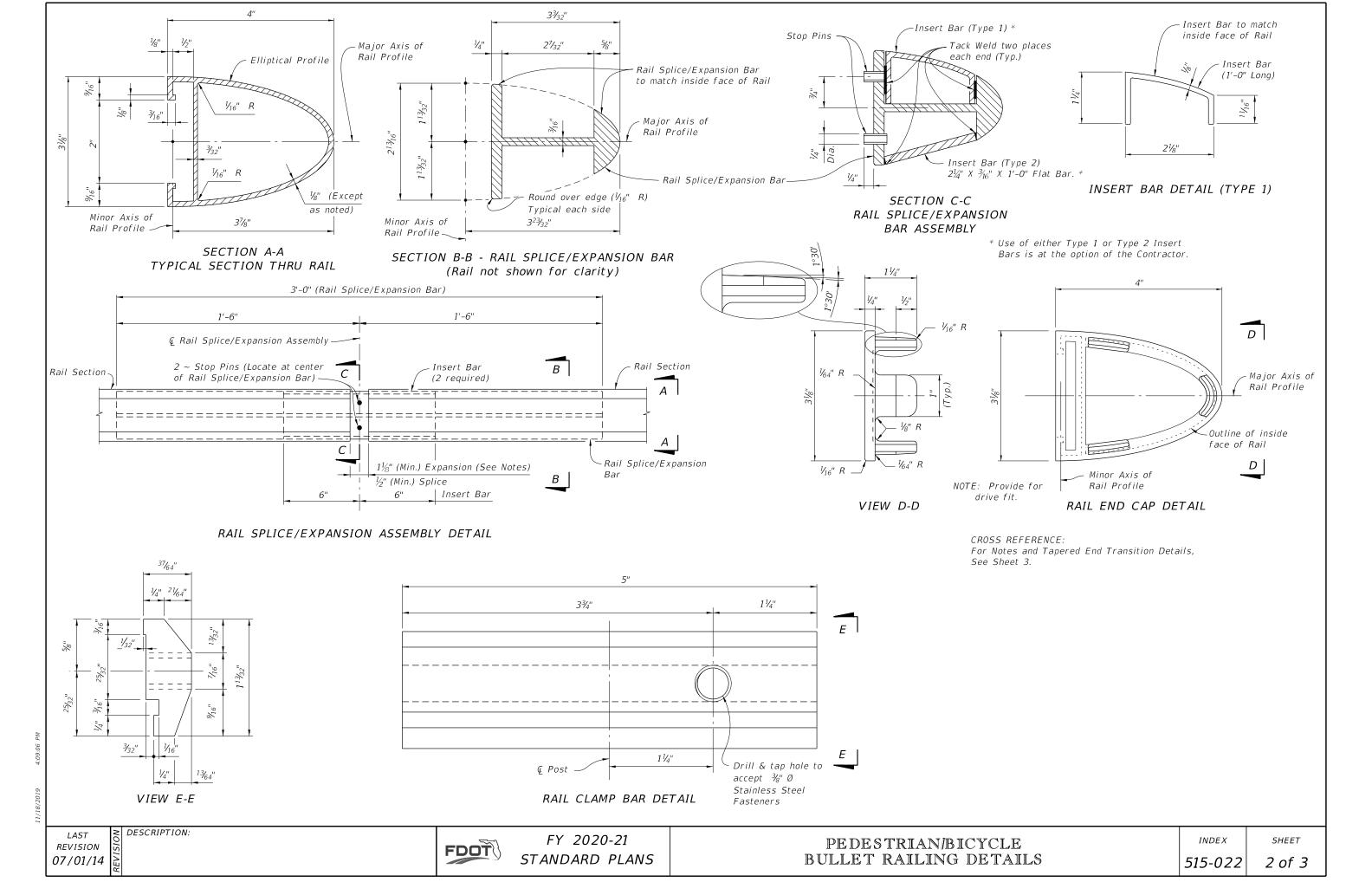
RAILING FOR TRAFFIC RAILING

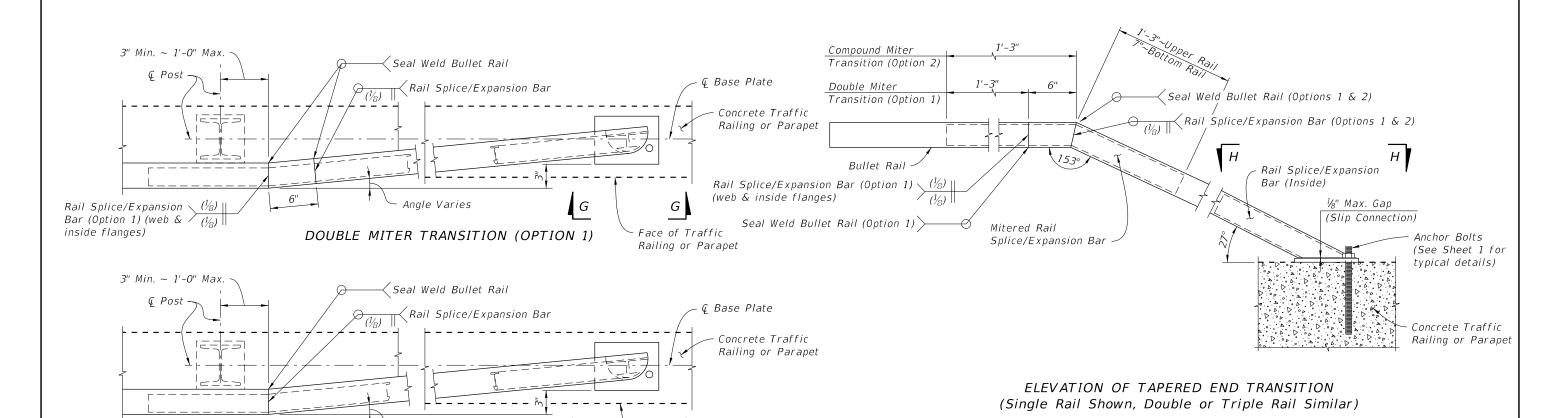
INDEX

SHEET 1 of 1

515-021







PARTIAL PLAN OF TAPERED END TRANSITIONS

COMPOUND MITER TRANSITION (OPTION 2)

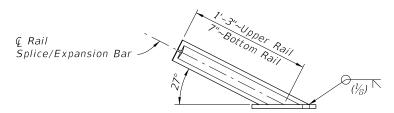
Angle Varies

G

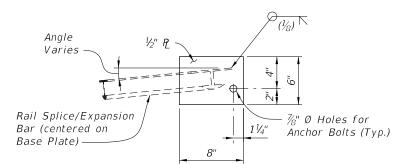
Face of Traffic

Railing or Parapet

(Single Rail Shown, Double or Triple Rail Similar)



VIEW G-G TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)



VIEW H-H TRANSITION BASE PLATE (Bullet Rail not shown for Clarity)

RAILING NOTES:

- Work this Index with Index 521-423, 521-427, 521-428, 521-820 and 515-021 and Specification Section 515.
- 2. Shop Drawings: Submit shop drawings prior to fabrication.
 - A. Include post and rail splice/expansion assembly location for curved alignments with radii < 40 feet and for all end terminations.
- Materials:
 - A. Supply Aluminum materials In accordance with Specification Section 965 and the following: Wrought Aluminum Post: ASTM B221, Alloy 6061-T6 or 6351-T5
 Rail End Cap: ASTM B26 sand cast aluminum alloy 356.0-F
 Plate and Bars: ASTM B209 Alloy 6061-T6
 Rails: ASTM B221 Alloy 6061-T6 or 6351-T5.
 Stop Pins: Press-fit aluminum or stainless steel pins or tubes
 - B. Stainless Steel Fasteners: ASTM F-593, Alloy Group 2 (316).
 - C. Bearing Pads: Plain or Fiber Reinforced meeting Specification Section 932 for Ancillary Structures.
- 4. Layout.
 - A. Posts shall be uniformly spaced with reasonable consistency.
 - B. Tapered End Transitions are required at the terminus of the approach ends of Bullet Railing mounted on a Traffic Railing. Bullet Railings on concrete parapets shielded by a traffic railing do not require Tapered End Transitions unless noted otherwise in the Plans.
 - C. Adjust post spacing's to avoid parapet obstacles, such as armor expansion plates, by 9 inches minimum.
 - D. Rails shall be continuous over a minimum of 3 posts, except that lengths less than 12 feet need only be continuous over 2 posts.
 - E. Space splices at 40 feet maximum. Splice all rails in a given railing section at about the same center line.
 - F. Provide rail expansion assemblies in panels between posts on either side of a bridge expansion joint. Rail expansion assemblies are similar to the rail splice assemblies with increased space at the expansion assembly to allow for movement equal to 1.5 times the bridge joint opening or 1" greater than the expected joint movement.
- 5. Installation:
 - A. Set rails near bridge expansion joints to allow for expected movement.
 - B. Cutting of reinforcing steel is permitted for post installed anchors.
- i. Payment: Includes the full cost of installed bullet railing. Cost of the Concrete Parapet or Traffic Railing is separate.

LAST REVISION 11/01/17

FDOT

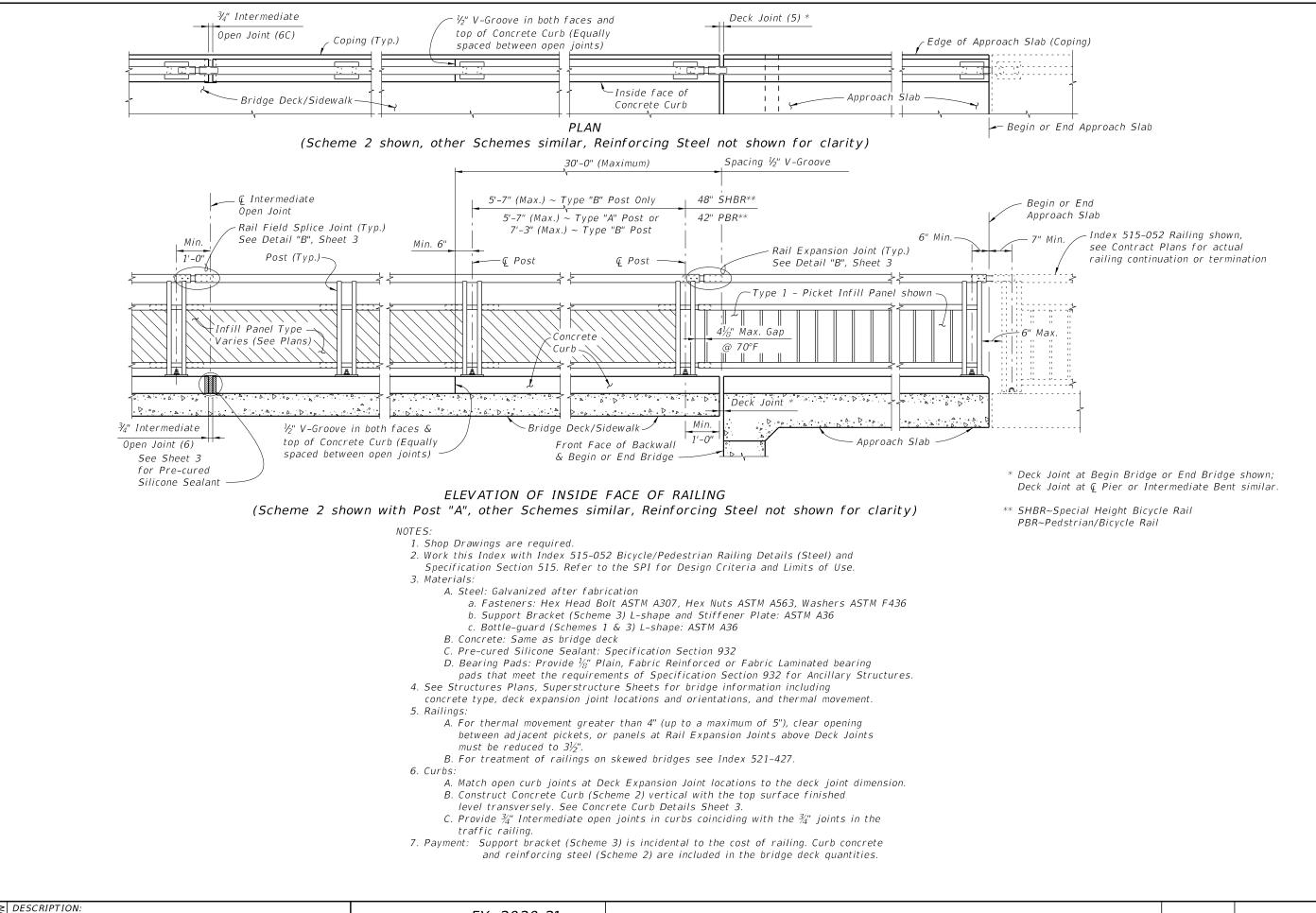
FY 2020-21 STANDARD PLANS

PEDESTRIAN/BICYCLE
BULLET RAILING DETAILS

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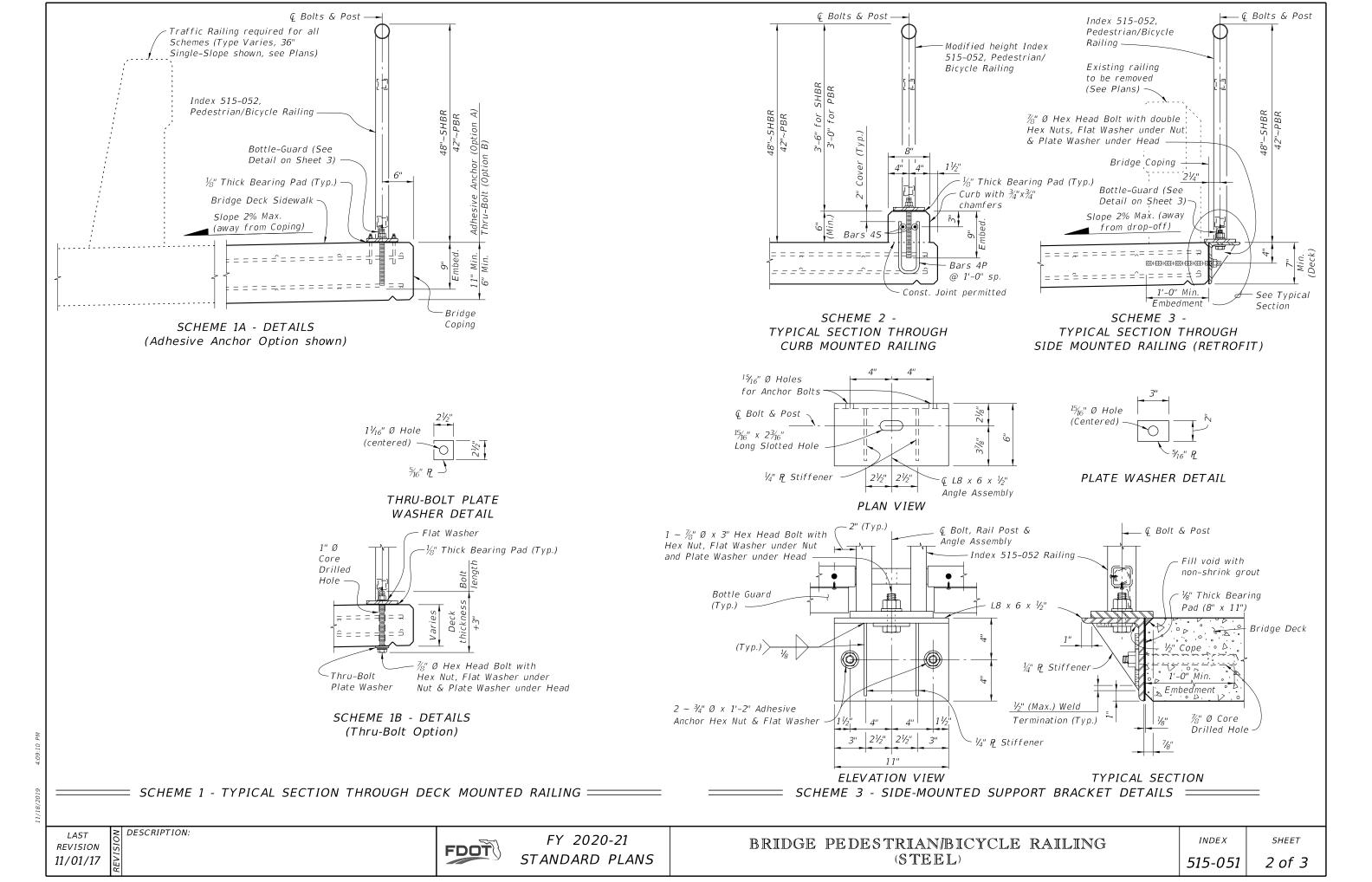
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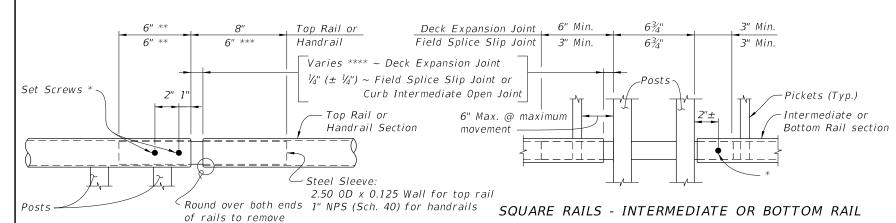


11/18/2019

LAST REVISION 11/01/17

FDOT



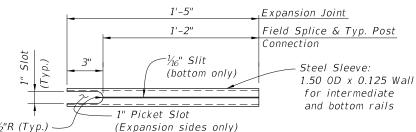


ROUND RAILS - TOP RAIL OR HANDRAIL

- * $\frac{1}{4}$ " Ø x $\frac{3}{4}$ " Pan Head Stainless Steel (Type 316 or 18–8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A $\frac{3}{4}$ " Ø plug weld may be substituted for the two set screws at expansion joints.
- ** Embedded length may be 4" for plug welded connection.
- *** Increase handrail sleeve embedment to 8" for Expansion Joint openings greater than 2".

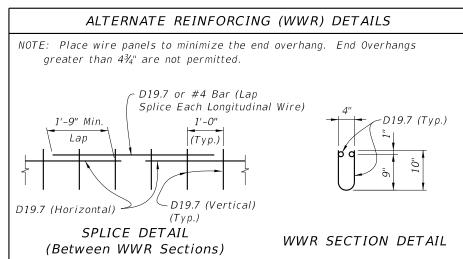
sharp edges (Typ.)

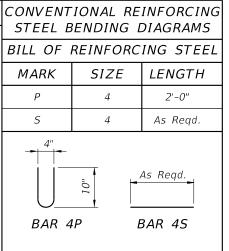
**** Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".



INTERMEDIATE OR BOTTOM RAIL - STEEL SLEEVE DETAIL (Bottom Side Shown)

= DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =





CURB REINFORCING STEEL NOTES:

ESTIMATED CONCRETE CURB

QUANTITIES (SCHEME 2)

UNIT

CY/LF

LB/LF

ITEM

Reinforcing Steel

Concrete

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".

QUANTITY

0.0124

4.01

5. Deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931 may be used in lieu of all Bars 4P and 4S.

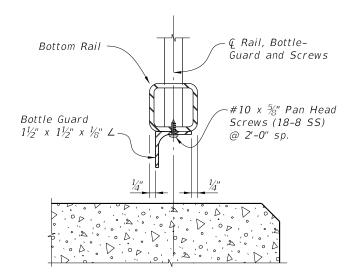
Pre-cured Silicone Sealant (4" wide)	6" Min. 34" Chamfer (Typ.)
over.	
[-	2"
ents and 4S.	

DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTE:

At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

SCHEME 2 - CONCRETE CURB DETAILS =

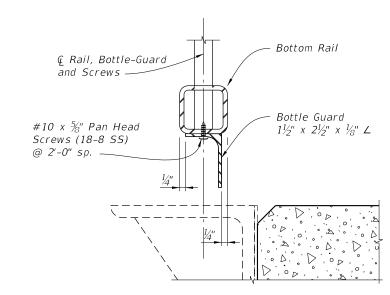


TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 1 - BOTTLE GUARD DETAIL =

CROSS REFERENCE:

See Sheet 1 for Bridge Railing Notes.



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL ===

LAST **REVISION** 11/01/16

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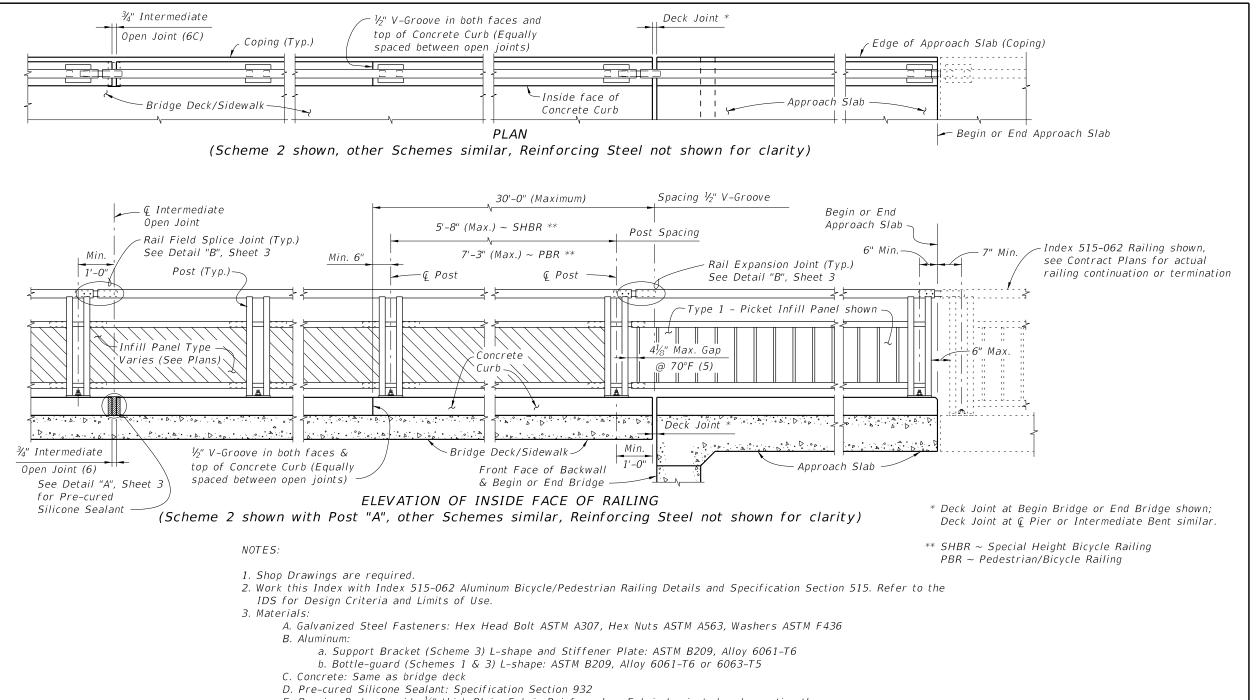
FY 2020-21 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING

INDEX

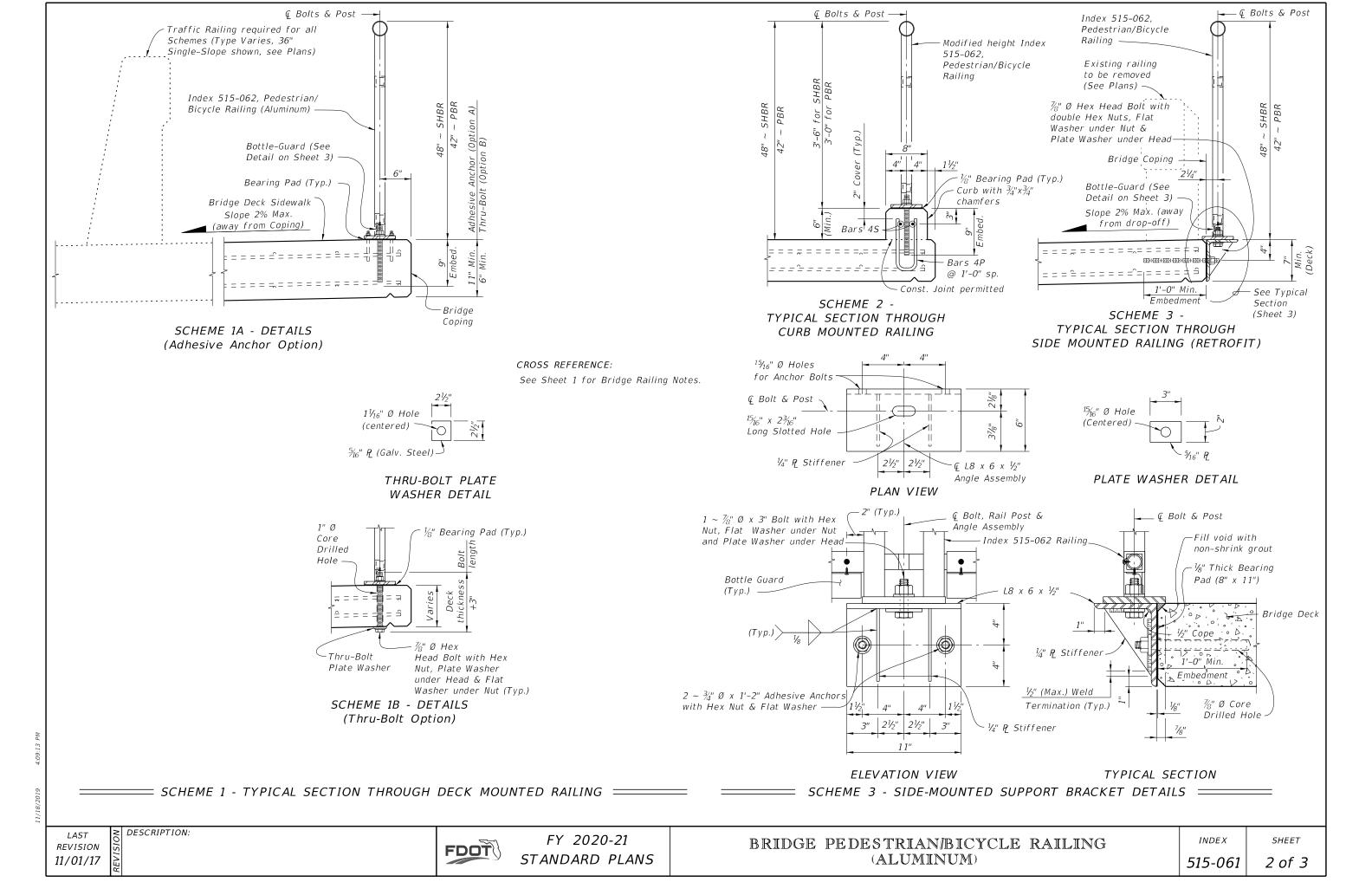
SHEET

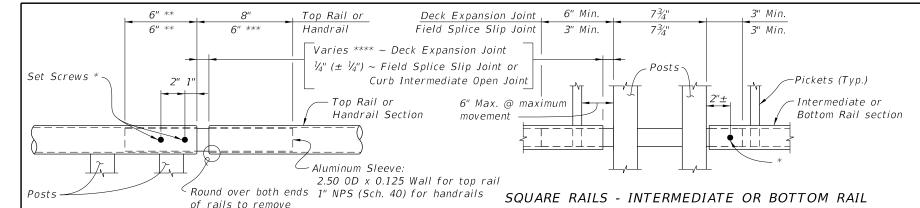
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- E. Bearing Pads: Provide $\frac{1}{2}$ " thick Plain, Fabric Reinforced or Fabric Laminated pads meeting the requirements of Specification Section 932 for Ancillary Structures.
- 4. See Structures Plans, Superstructure Sheets for bridge information including concrete type, deck expansion joint locations and orientations, and thermal movement.
- 5. Railings:
 - A. For thermal movement greater than 4" (up to a maximum of 5"), clear opening between adjacent pickets, or panels at Rail Expansion Joints above Deck Joints must be reduced to $3\frac{1}{2}$.
 - B. For treatment of railings on skewed bridges see Index 521-427.
- 6. Curbs:
 - A. Match open curb joints at Deck Expansion Joint locations to the deck joint dimension.
 - B. Construct Concrete Curb (Scheme 2) vertical with the top surface finished level transversely. See Concrete Curb Details Sheet 3.
 - C. Provide $\frac{3}{4}$ " Intermediate open joints in curbs coinciding with the $\frac{3}{4}$ " joints in the traffic railing.
- 7. Payment: Support bracket (Scheme 3) is incidental to the cost of railing. Curb concrete and reinforcing steel (Scheme 2) are included in the bridge deck quantities.

LAST **REVISION** 11/01/17



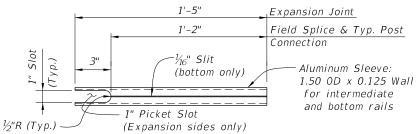


ROUND RAILS - TOP RAIL OR HANDRAIL

- * ¼" Ø x ¾" Pan Head Aluminum (Alloy 7075-T73) or Stainless Steel (Type 316 or 18-8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A ¾" Ø plug weld may be substituted for the two set screws at expansion joints.
- ** Embedded length may be 4" for plug welded connection.
- *** Increase handrail sleeve embedment to 8" for Expansion Joint openings greater than 2".

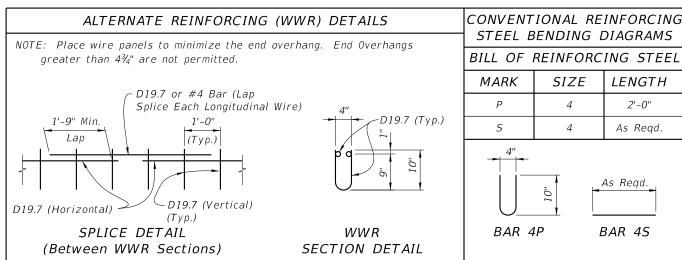
sharp edges (Typ.)

**** Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".



INTERMEDIATE OR BOTTOM RAIL - ALUMINUM SLEEVE DETAIL (Bottom Side Shown)

DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =



CURB REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.

 2. All reinforcing steel at the open injects shall have a 2" mining.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Deformed WWR meeting the requirements of Specifications Section 931 may be used in lieu of all Bars 4P and 4S.

re-cured Sealant (4" r.	6" Min.	ł	" Chamfer	(Тур

DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

ESTIMATED CONCRETE CURB QUANTITIES (SCHEME 2) ITEM UNIT QUANTITY

ITEM UNIT QUANTITY

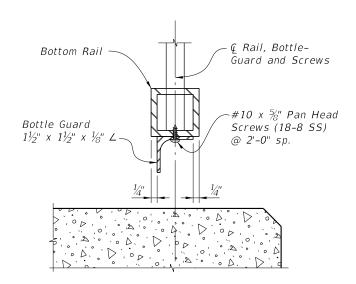
Concrete CY/LF 0.0124

Reinforcing Steel LB/LF 4.01

INTERMEDIATE JOINT SEAL NOTE:

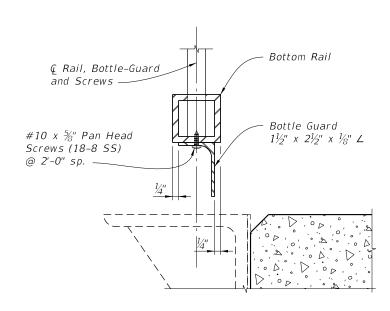
At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

SCHEME 2 - CONCRETE CURB DETAILS =



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 1 - BOTTLE GUARD DETAIL =



TYPICAL SECTION THROUGH BOTTOM RAIL
(Post Not Shown for Clarity)

== SCHEME 3 - BOTTLE GUARD DETAIL =

IAST | DESCRIPTION:

REVISION

11/01/16

FDOT

FY 2020-21 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

INDEX

SHEET

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

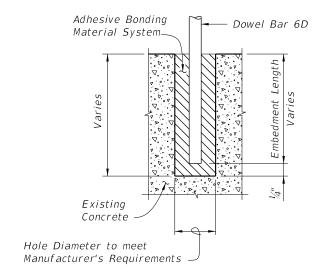
BRIDGES ON CURVED ALIGNMENTS: The details presented in this Standard are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing along the entire length of the bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

GUARDRAIL: See Index 536-001 for guardrail component details, geometric layouts and associated notes not fully detailed

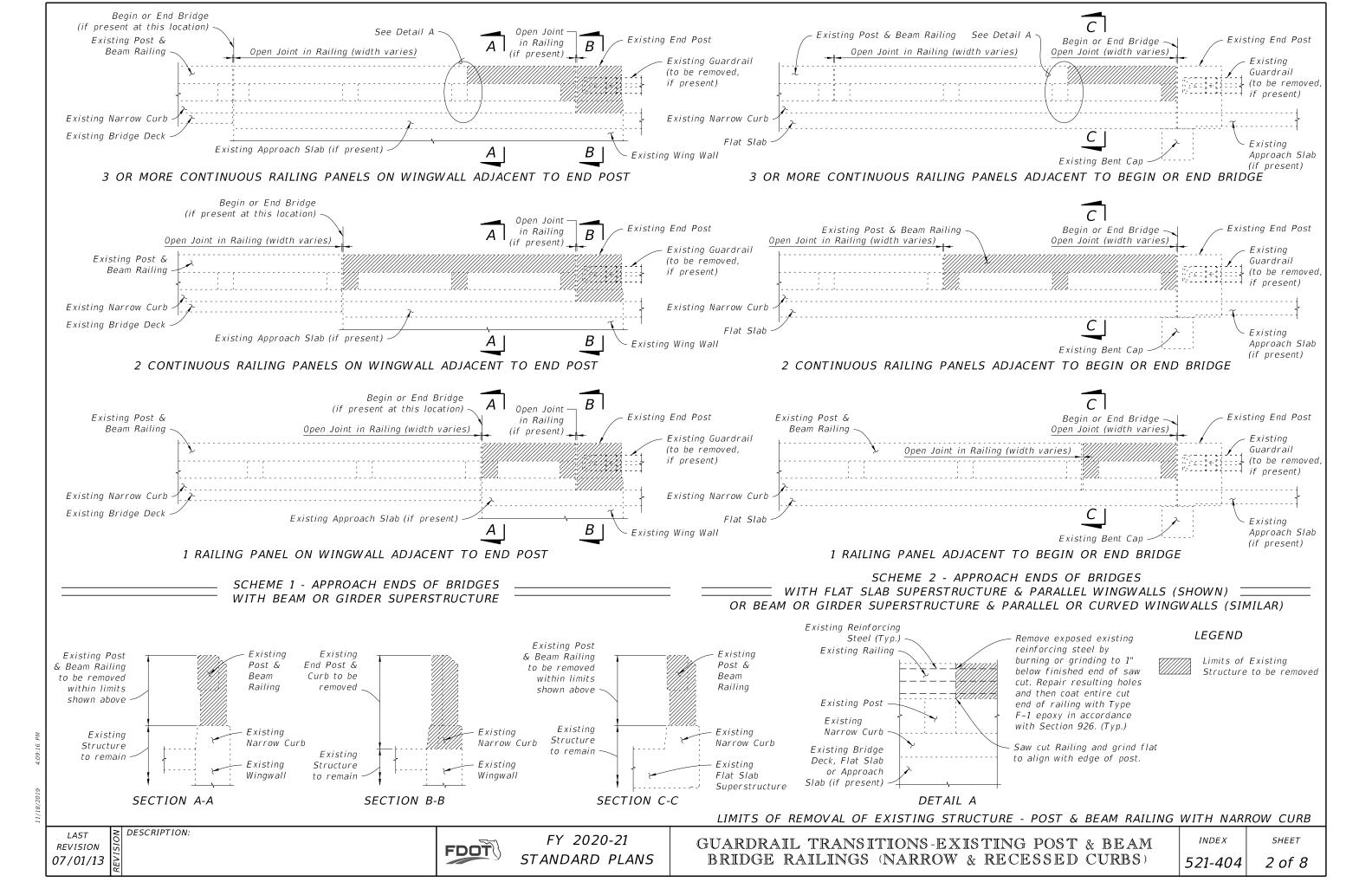
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise individual decals of letters

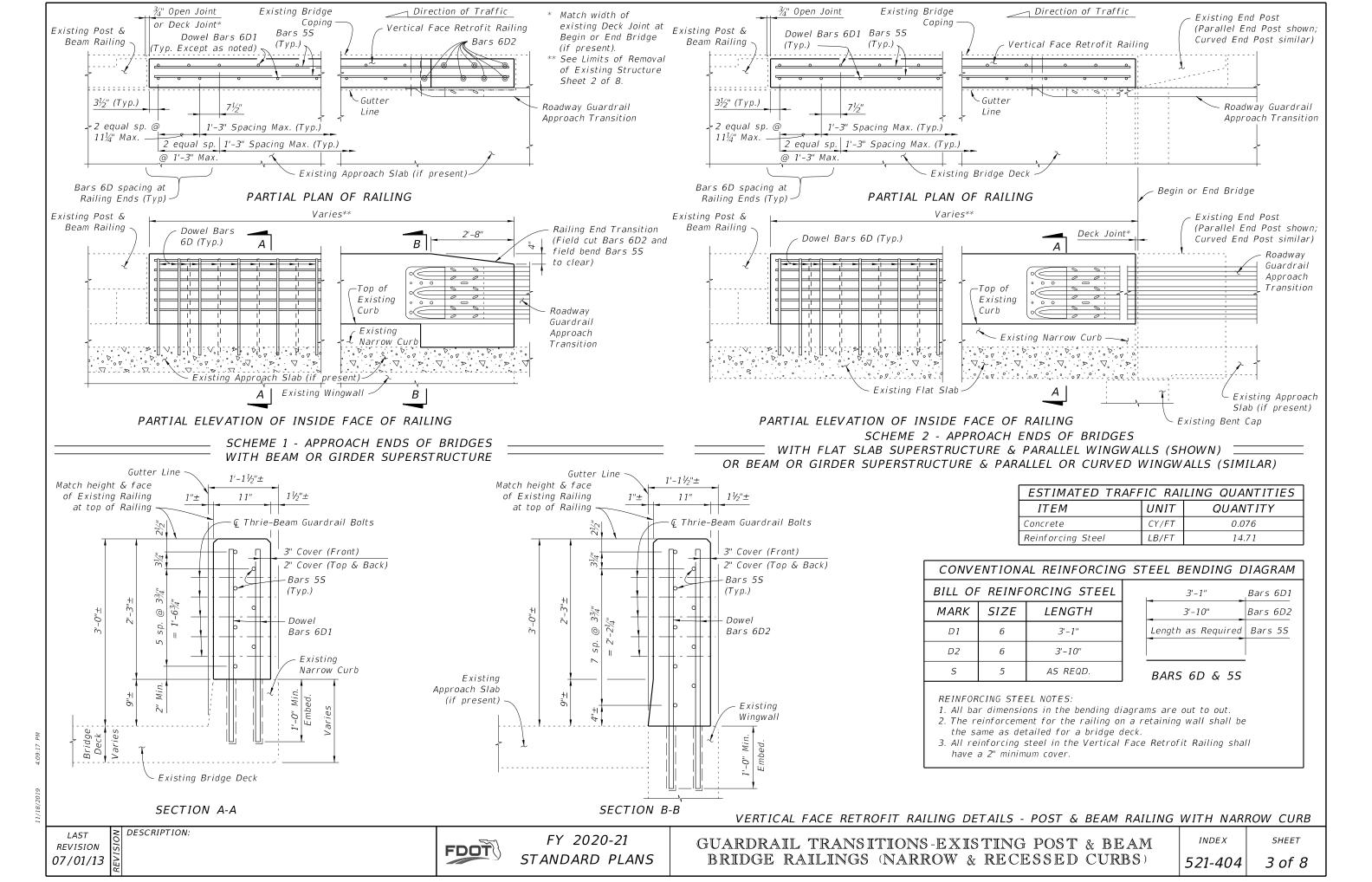
PAYMENT: Concrete Traffic Railing-Bridge Retrofit - Post & Beam Railing (EA) includes all material and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railing. Guardrail Approach Transition to rigid Barriers (EA) includes transition block, and necessary hardware to complete the Guardrail transitions shown.

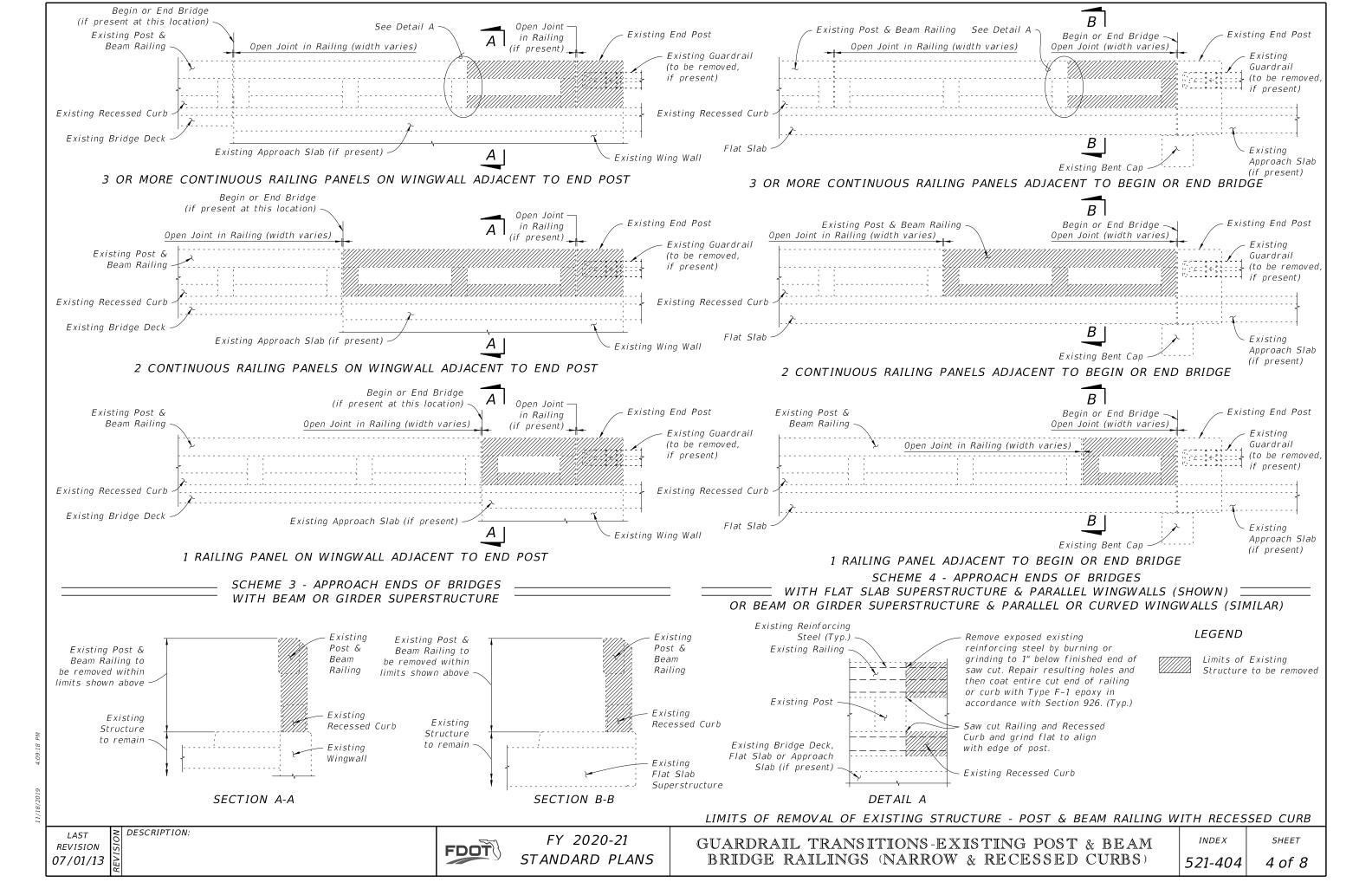


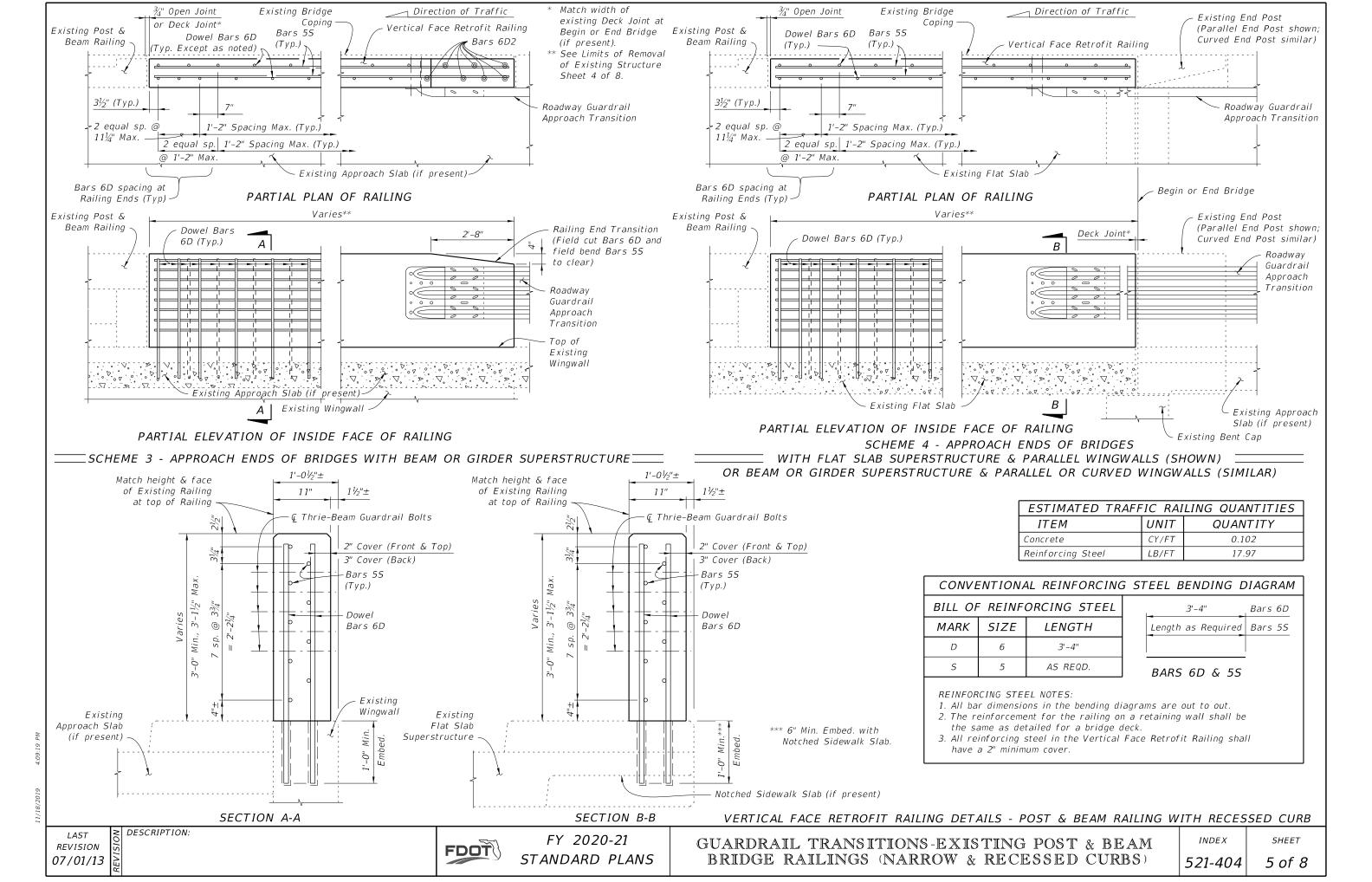
DOWEL DETAIL

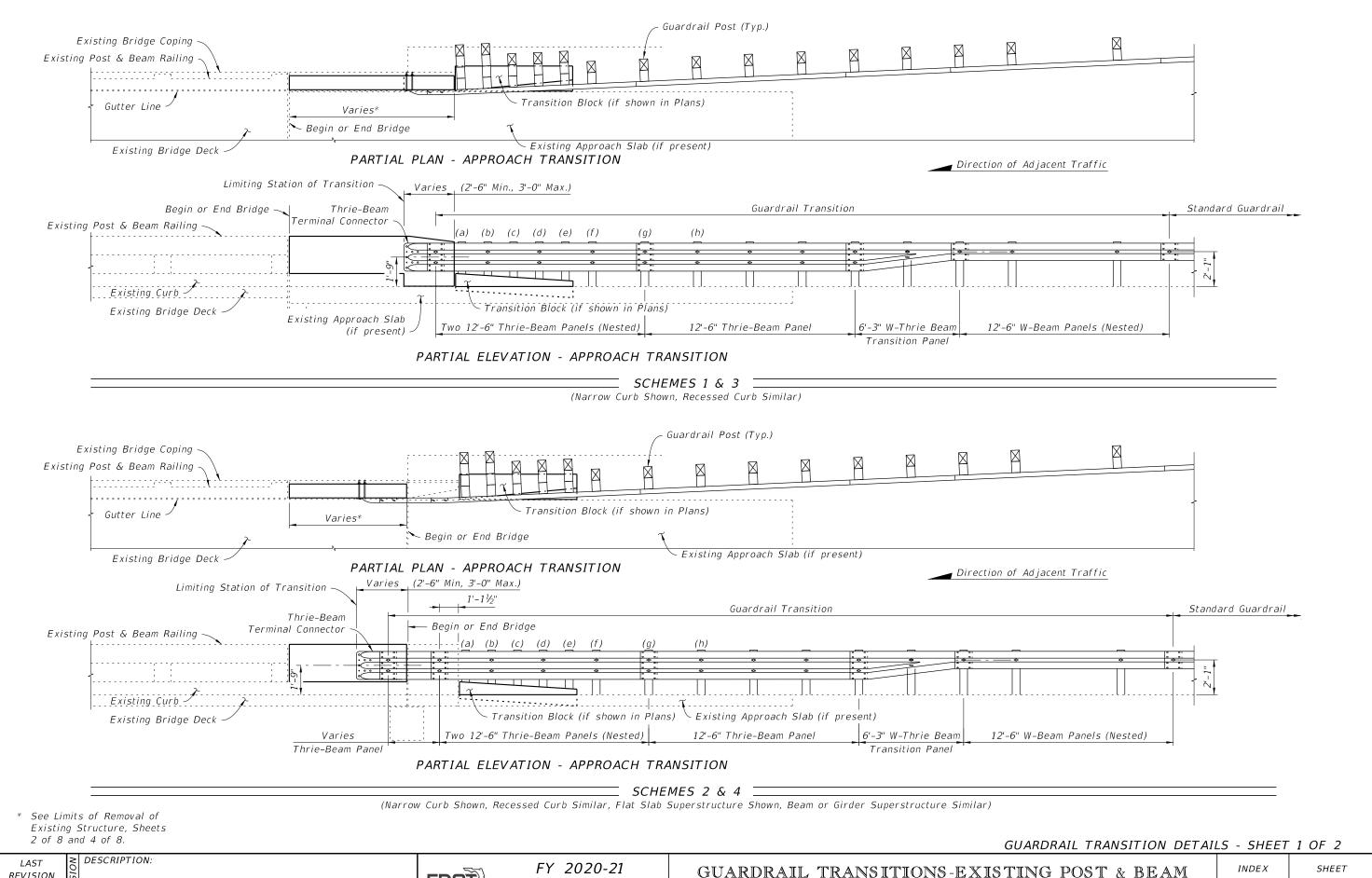
Note: Shift dowel holes to clear if the existing reinforcement is encountered.











REVISION 07/01/14

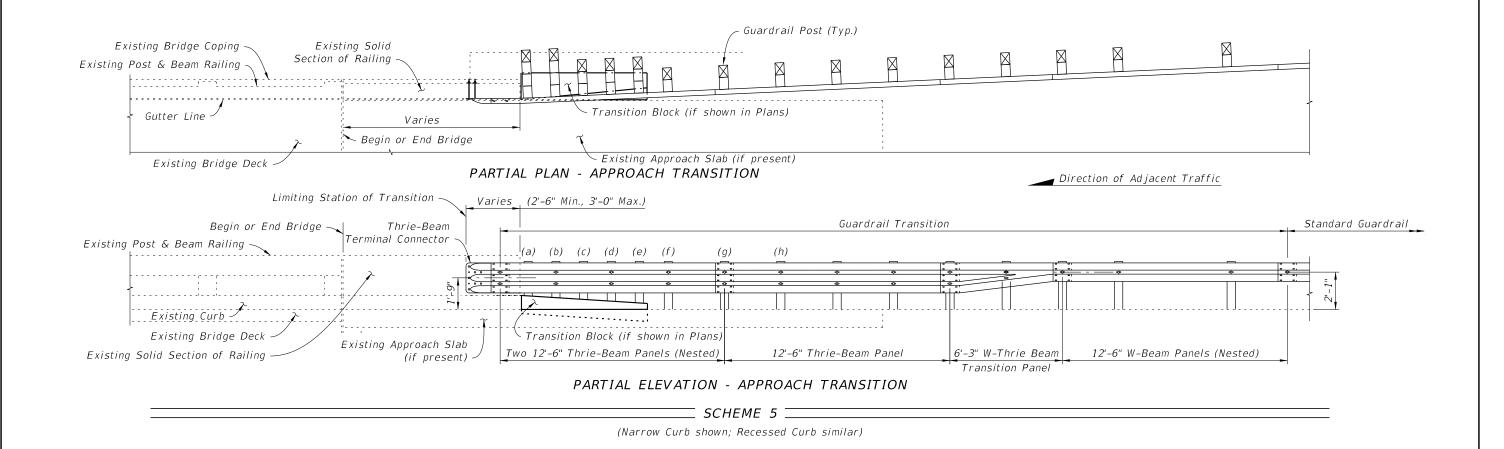
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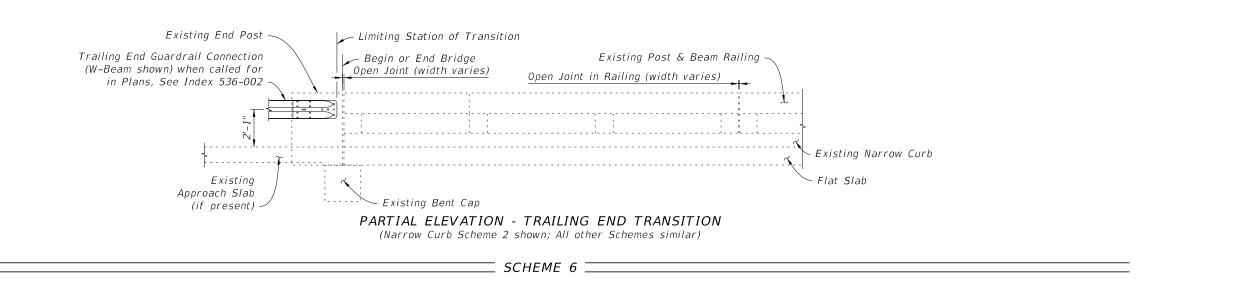
STANDARD PLANS

BRIDGE RAILINGS (NARROW & RECESSED CURBS)

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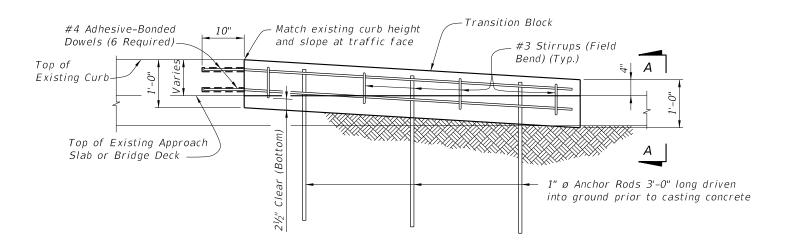


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GUARDRAIL TRANSITION DETAILS - SHEET 2 OF 2

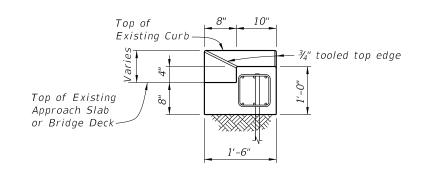
SHEET

PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

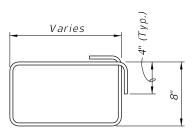


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES PER TRANSITION BLOCK		
ITEM	UNIT	QUANTITY
Concrete Class II (Bridge Deck)	CY	0.4
Reinforcing Steel	LB	61



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

ADHESIVE-BONDED DOWELS: Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

Shift bars (as needed) to install six dowels into existing bridge or approach slab mounted curb.

DESCRIPTION:

GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

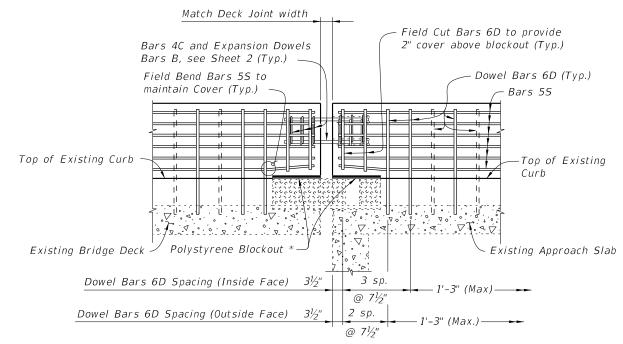
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install barrier delineators on top of the Traffic Railing along the entire length of bridge 2" from the face on the traffic side in accordance with Specification Section 705. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Concrete Traffic Railing – Bridge Retrofit – Post & Beam Railing (each) includes all materials and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railings. Guardrail Approach Transition to Rigid Barriers (EA) includes all transition blocks, and necessary hardware to complete the Guardrail transitions shown.

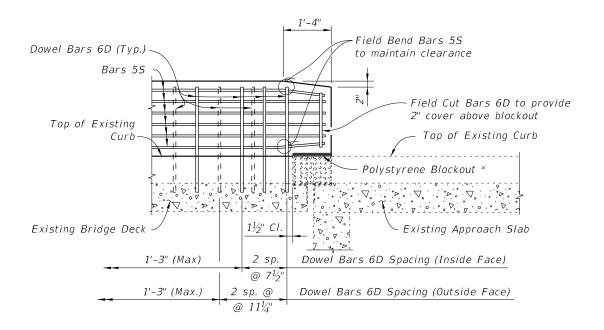
ESTIMAT	TED TRAF	FIC RAILING QU	<i>JANTITIES</i>
ITEM UNIT	UNIT	QUANTITY	
II LIVI	ONT	9" Curb	Increment
Concrete	CY/FT	0.064	0.003 per in. height
Reinforcing Steel	LB/FT	13.27	0.10 per in. length

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.)



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEMES 2 THRU 5

* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE
JOINT AT BEGIN OR END BRIDGE - SCHEME 1
(Guardrail Transition not shown for clarity)

LAST REVISION 11/01/19

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

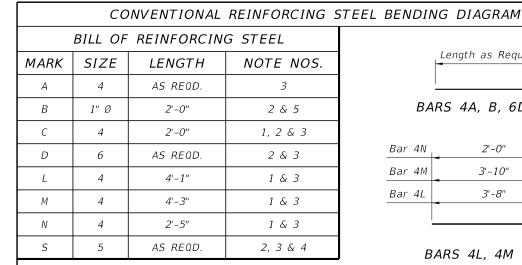
GUARDRAIL TRANSITIONS - EXISTING
POST & BEAM BRIDGE RAILINGS (WIDE CURBS)

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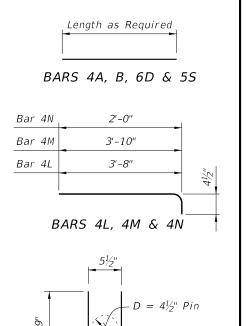
SHEET

0100



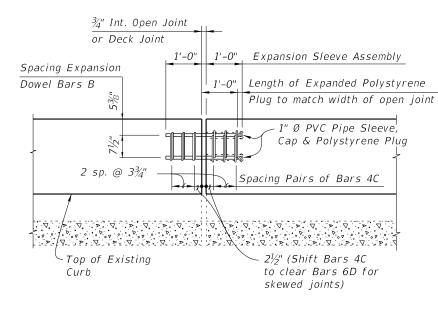
REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.

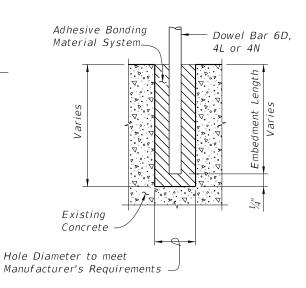


BARS 4C

(12 required per open joint)



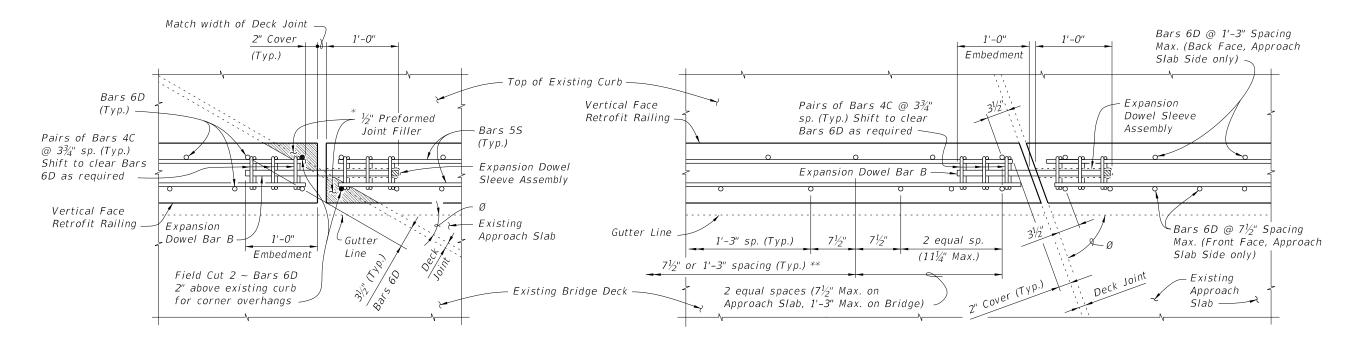
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



DOWEL DETAIL

Dowel Installation Note: Shift dowel holes to clear if the existing reinforcement is encountered.

* ½" Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.



SKEW DETAIL =

1/18/2019 4:0

LAST REVISION 07/01/13

DESCRIPTION:

FDOT

PARTIAL PLAN OF RAILING (SKEW ANGLE Ø LESS THAN 70°)

FY 2020-21 STANDARD PLANS

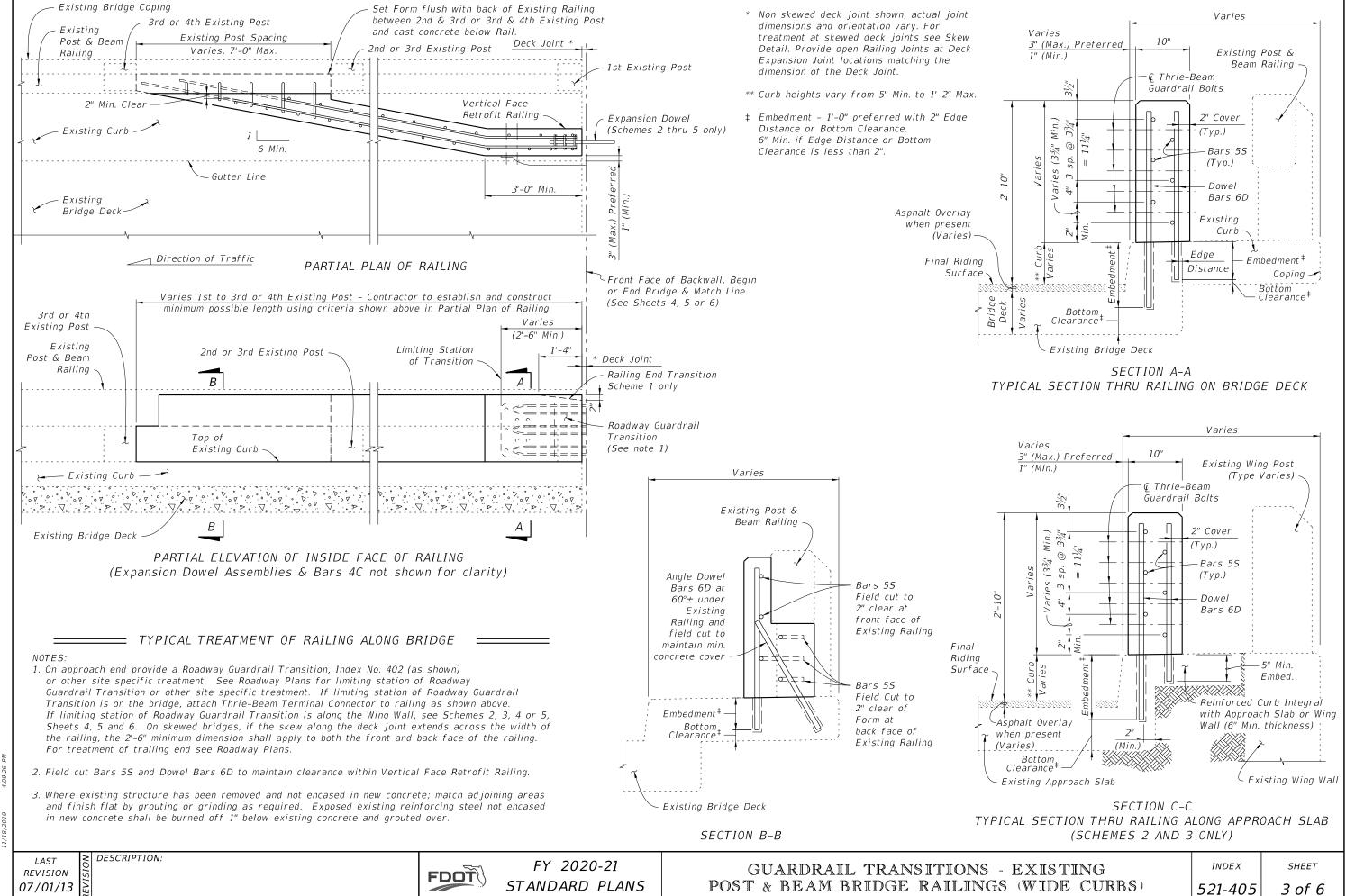
GUARDRAIL TRANSITIONS - EXISTING
POST & BEAM BRIDGE RAILINGS (WIDE CURBS)

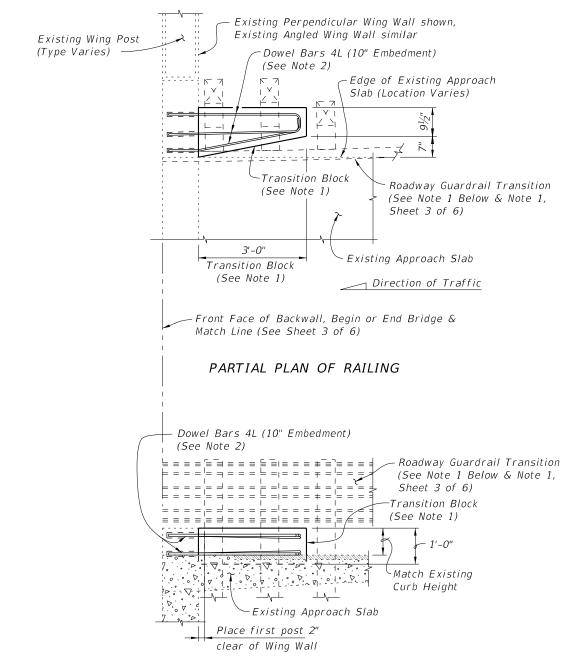
PARTIAL PLAN OF RAILING (SKEW ANGLE Ø = 70° OR GREATER)

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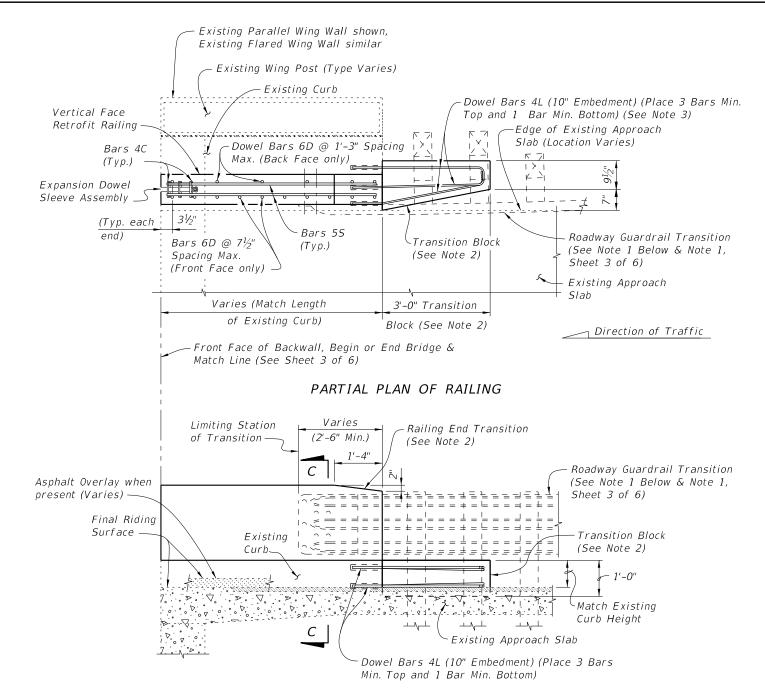


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

> ====== SCHEME 2 ==== RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 3 of 6. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

REVISION 07/01/13

DESCRIPTION:

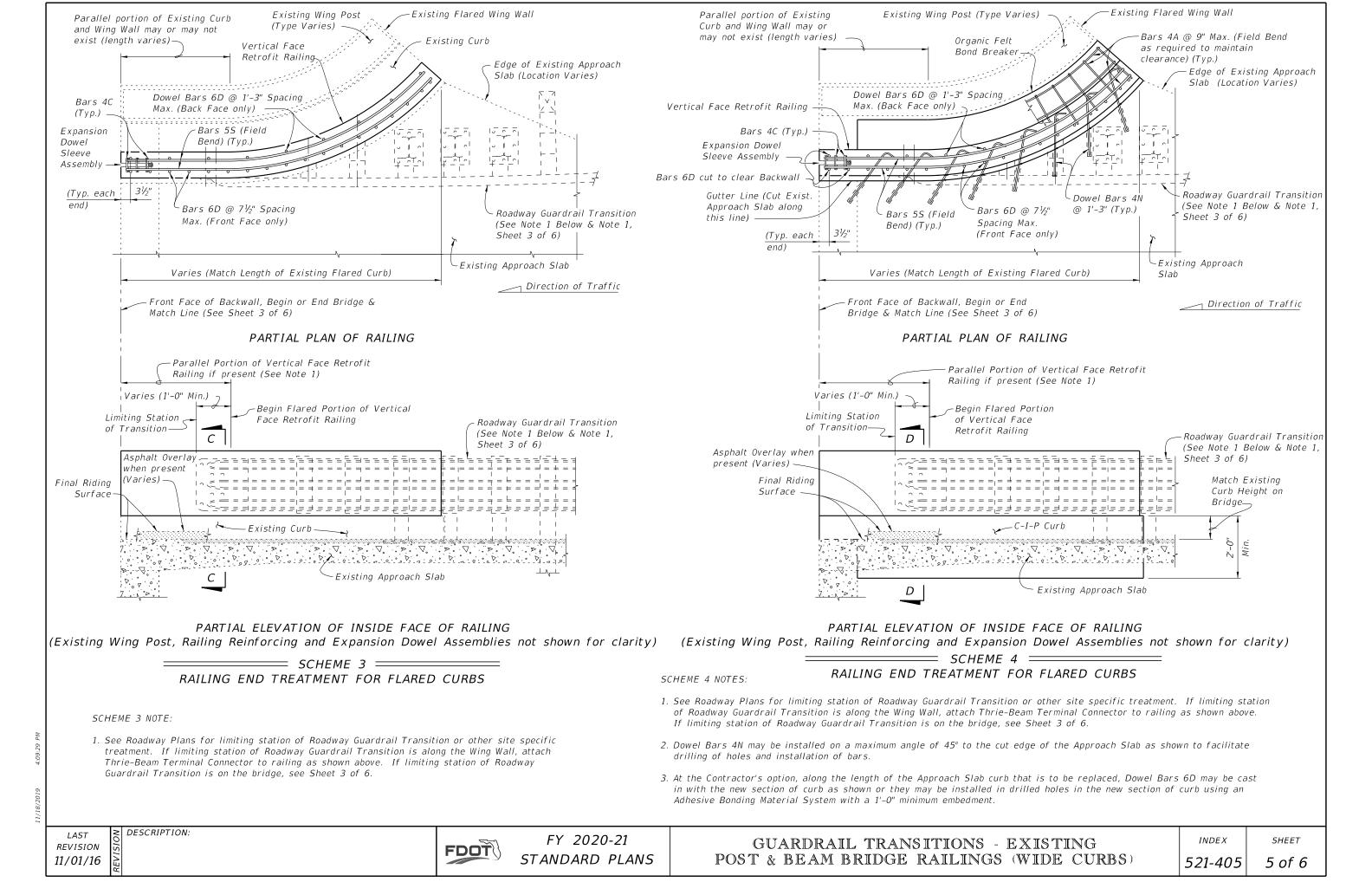
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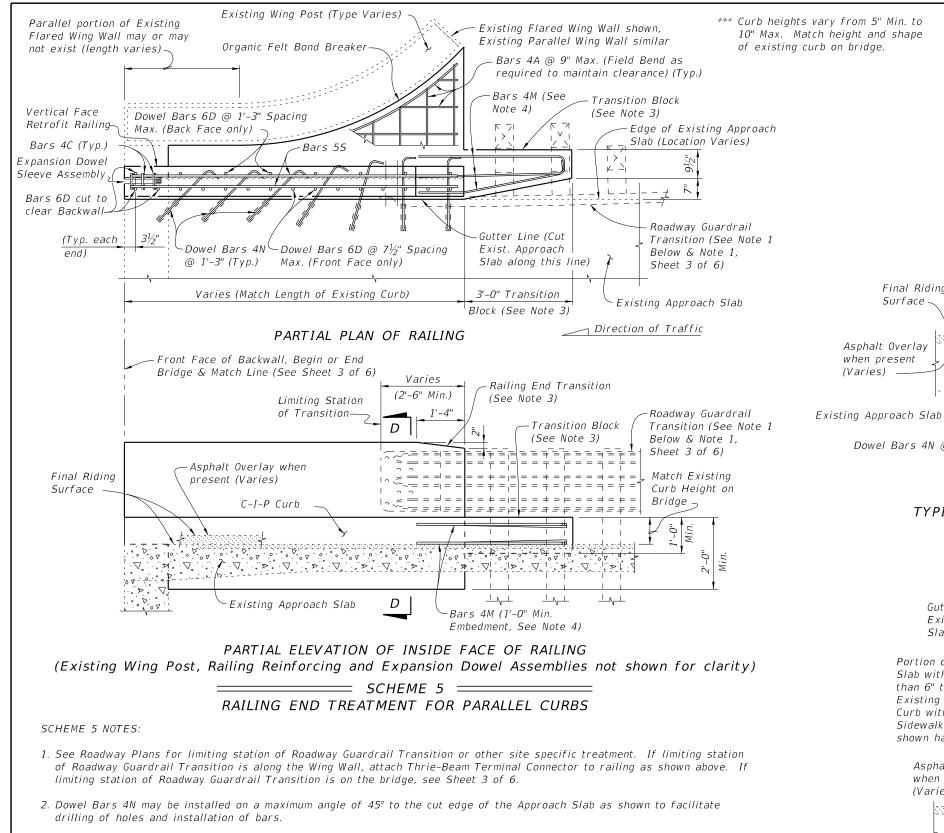
FY 2020-21 STANDARD PLANS GUARDRAIL TRANSITIONS - EXISTING

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SHEET

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- Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment

(SCHEMES 4 AND 5 ONLY) GUARDRAIL TRANSITIONS - EXISTING

REVISION 11/01/16

DESCRIPTION:

FY 2020-21 STANDARD PLANS

INDEX SHEET POST & BEAM BRIDGE RAILINGS (WIDE CURBS) 6 of 6 *521-405*

TYPICAL SECTION THRU EXISTING APPROACH

SLAB AND END BENT WING WALL SHOWING LIMITS OF REMOVAL

Wing Wall

Varies

2" Clear

Bars 55

Dowel Bars 6D

(See Note 5)

(Typ.)

(Typ.)

Existing Wing Post

(Type Varies)

Organic Felt

Bond Breaker

Bars 4A @ 9" Max

2" Min. Clear. Top

and Sides, 4" Min.

Clear. Bottom -

Bars 4A @ 9" Max., Min. 3 full length bars

required Top & Bottom (Field Bend to clear) (Typ.)

Varies (1'-2" Min.)

Guardrail Bolts

Varies

1" Min.

Final Riding

Dowel Bars 4N @ 1'-3" (Typ.)

Gutter Line (Cut

Existing Approach

Portion of Existing Approach

Slab with Integral Curb less than 6" thick or portion of

Existing Approach Slab and

Curb with Floating Detached Sidewalk to be removed

shown hatched.

(Varies)

Existing Approach Slab

Asphalt Overlay when present

Slab along this line) _

Surface

Asphalt Overlay

when present

(Varies)

3" (Max.) Preferred

@ 111 (3¾"

'Embedmen

SECTION D-D

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

Existing Wing Post

Varies (1'-2" Min.)

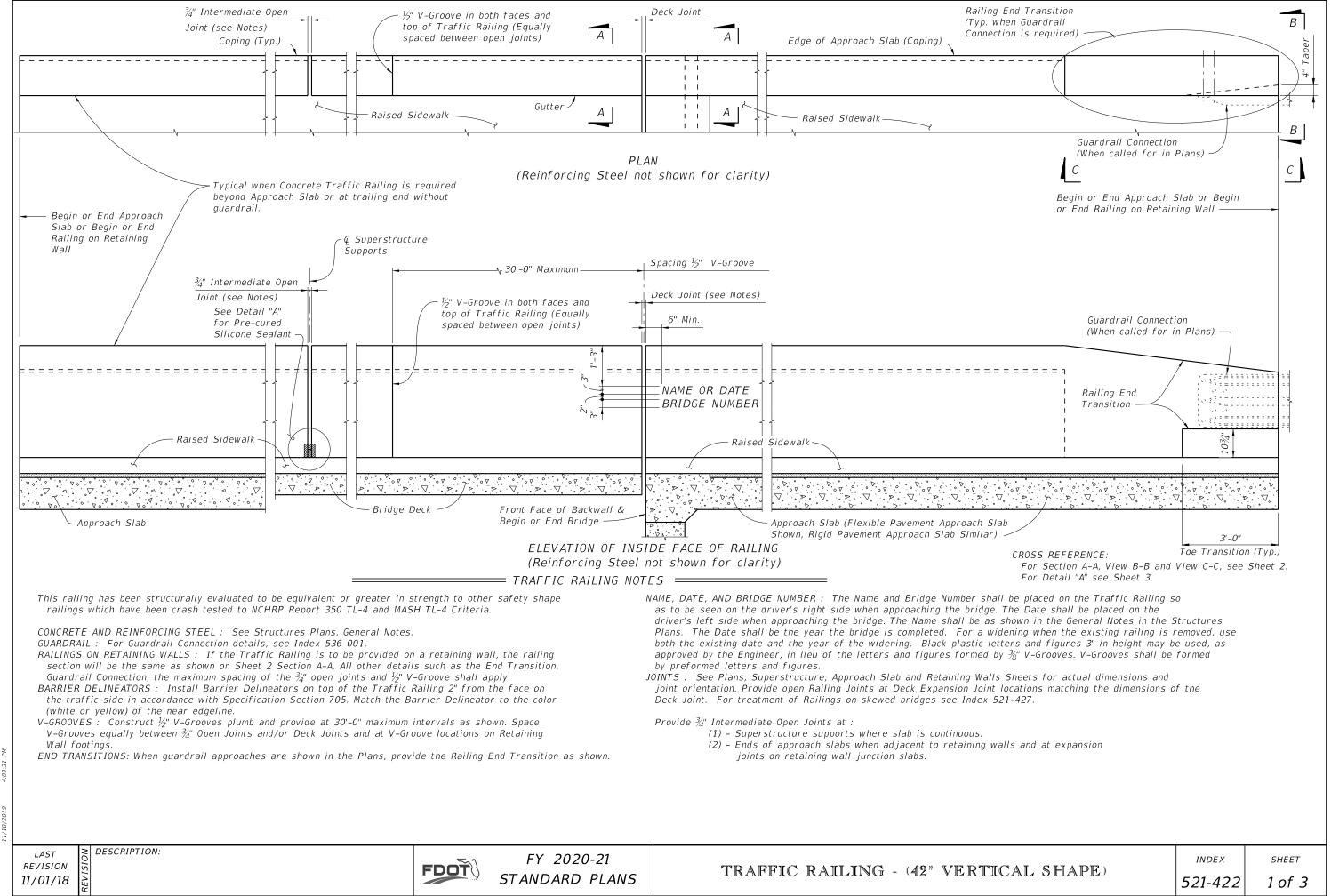
(Type Varies)

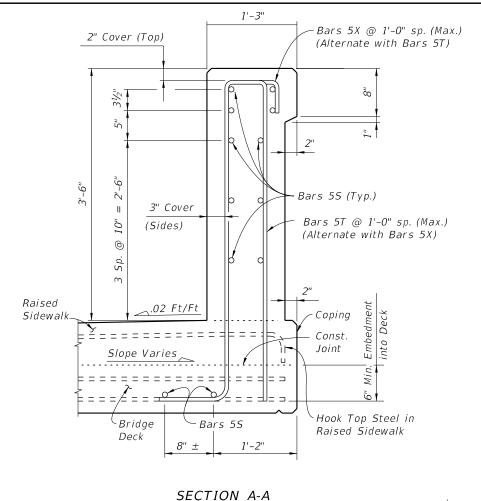
Varies

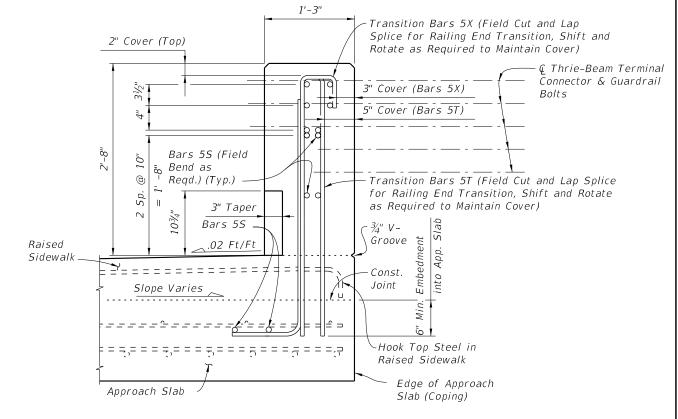
sp.

3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End

4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.





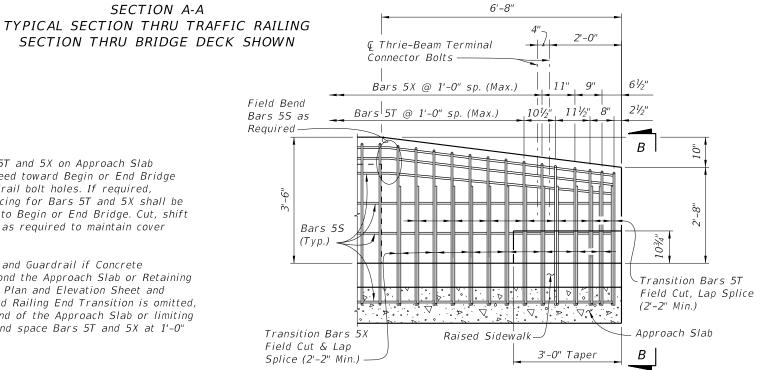


CROSS REFERENCE:

For location of Section A-A, View B-B

and View C-C, see Sheet 1.

VIEW B-B (END VIEW OF TRAFFIC RAILING END TRANSITION) (Approach Slab shown, Retaining Wall Junction Slab similar)



VIEW C-C RAILING END TRANSITION (Guardrail Not Shown For Clarity)

NOTES:

- 1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.
- 2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)

REVISION 11/01/17

DESCRIPTION:

FDOT

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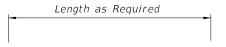
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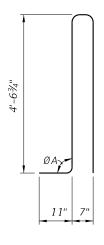
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK	LENGTH			
S	5	As Reqd.		
Т	5	10'-8"		
Χ	5	6'-9"		

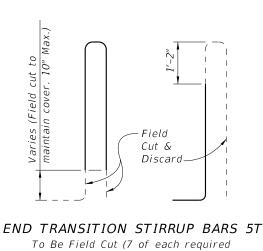
ROADWAY	ØA		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	83°	
6% to 10%	84°	96°	



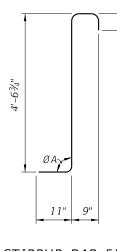
BAR 5S

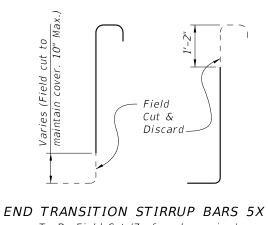


STIRRUP BAR 5T



per Railing End Transition)



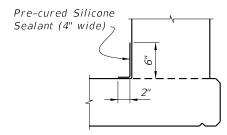


STIRRUP BAR 5X

To Be Field Cut (7 of each required per Railing End Transition)

REINFORCING STEEL NOTES:

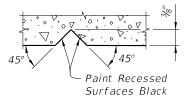
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 4'-6¾" vertical dimension shown for Bars 5T and 5X is based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slope vary from the above amounts, adjust this dimension accordingly to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above with $\emptyset A = 90^{\circ}$.
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



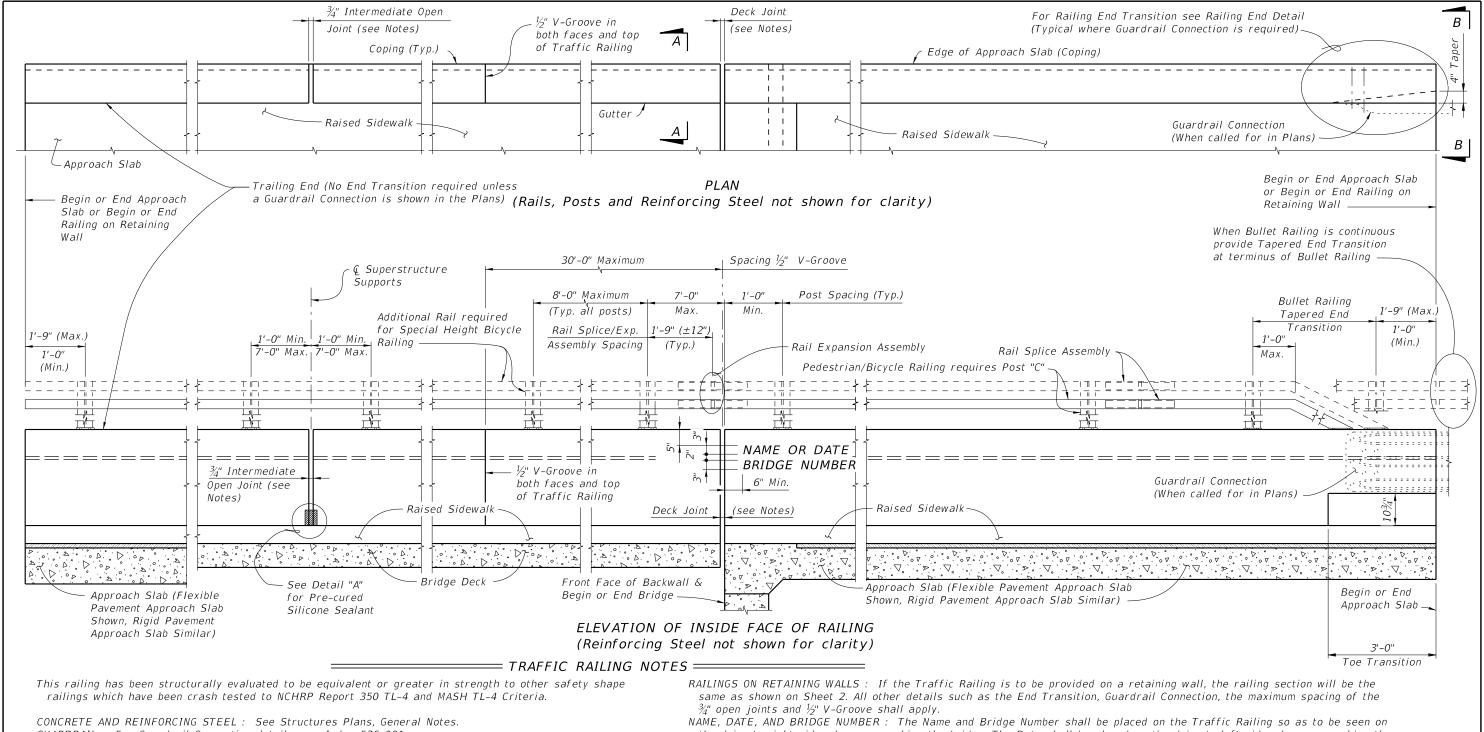
SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.145
Reinforcing Steel	LB/LF	30.68

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)

11/18/2019 4:09

DESCRIPTION:



GUARDRAIL: For Guardrail Connection details, see Index 536-001.

PEDESTRIAN/BICYCLE RAILING AND SPECIAL HEIGHT BICYCLE RAILING DETAILS: See Index 515-022 for Post, Rail and Rail Splice/Expansion Assembly fabrication and installation Details and Notes. V-GROOVES: Construct ½" V-Grooves plumb. Space V-Grooves equally between ¾" Open Joints

and/or Deck Joints and at V-Groove locations on Retaining Wall footings.

BARRIER DELINEATORS: Install Barrier Delineators on top of the Traffic Railing 2" from the face on the traffic side in accordance with Specification Section 705. Match the Barrier Delineator to the color (white or vellow) of the near edgeline.

END TRANSITION: When guardrail approaches are shown in the plans, provide Railing End Transition.

the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Name shall be as shown in the General Notes of the Structures Plans. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the year of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by $rac{3}{6}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures.

OPEN JOINTS: See Structures Plans, Superstructure, Approach Slab Sheets and Retaining Walls for actual dimensions and joint orientation. Provide open Traffic Railing Joints at Deck Expansion Joint locations matching the dimensions of the Deck Joint. For treatment of Railings on skewed bridges see Index 521-427.

Provide $\frac{3}{4}$ " Intermediate Open Joints at :

- (1) Superstructure supports where slab is continuous.
- (2) Ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.

CROSS REFERENCE: For Section A-A and View B-B, see Sheet 2. For Detail "A" see Sheet 3

REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (32" VERTICAL SHAPE)

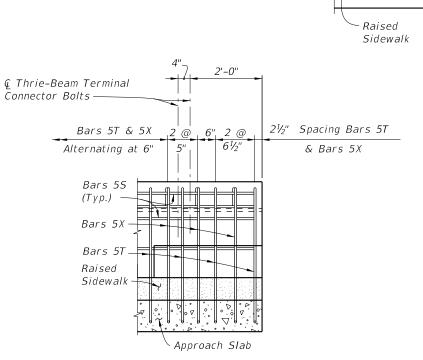
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SHEET 1 of 3

SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING (Section Thru Bridge Deck shown)

NOTES:

- 1. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.
- 2. Omit Railing End Transition and Guardrail if Concrete Traffic Railing is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)



VIEW B-B APPROACH SLAB END VIEW OF TRAFFIC RAILING

Bars 5S

1'-0"

1'-1"

- Bars 5X @ 1'-0" sp. (Max.) (Alternate with

Bars 5T) (See Note 1)

Hook Top Steel in

Edge of Approach

Raised Sidewalk

Slab (Coping)

♀ Thrie-Beam Terminal

Connector & Guardrail

Bolts

Bars 5S (Field Bend as

Bars 5T @ 1'-0" sp. (Max.)

(Alternate with Bars 5X)

Required) (Typ.)

(See Note 1)

Additional Rail required for

Railing)

Bicycle

Raili

(Pedestrian/Bicycle

Special Height Bicycle Railing

Pedestrian/Bicycle Railing

2" Cover (Top)

3" Taper

.02 Ft/Ft

Slope Varies

Approach

Slab

CROSS REFERENCE: For location of Section A-A and View B-B see Sheet 1.

Const. Joint

NOTE: For Bullet Railing Details, see Index 515-022.

RAILING END DETAIL (Guardrail Not Shown For Clarity)

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (32" VERTICAL SHAPE)

INDEX

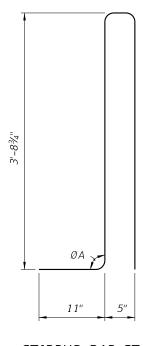
SHEET

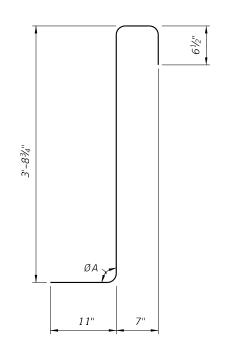
521-423 2 of 3

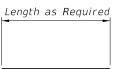
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK	LENGTH			
S	5	As Reqd.		
Т	5	9'-0"		
Х	5	5'-10"		

ROADWAY	ØA		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	93°	
6% to 10%	84°	96°	







BAR 5S

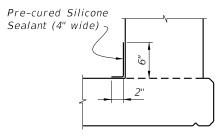
STIRRUP BAR 5T

DESCRIPTION:

STIRRUP BAR 5X

REINFORCING STEEL NOTES:

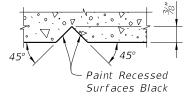
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 3'-8¾" vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into
- 3. The reinforcement for the railing on a Retaining Wall shall be the same as detailed with $\emptyset A = 90^{\circ}$.
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.

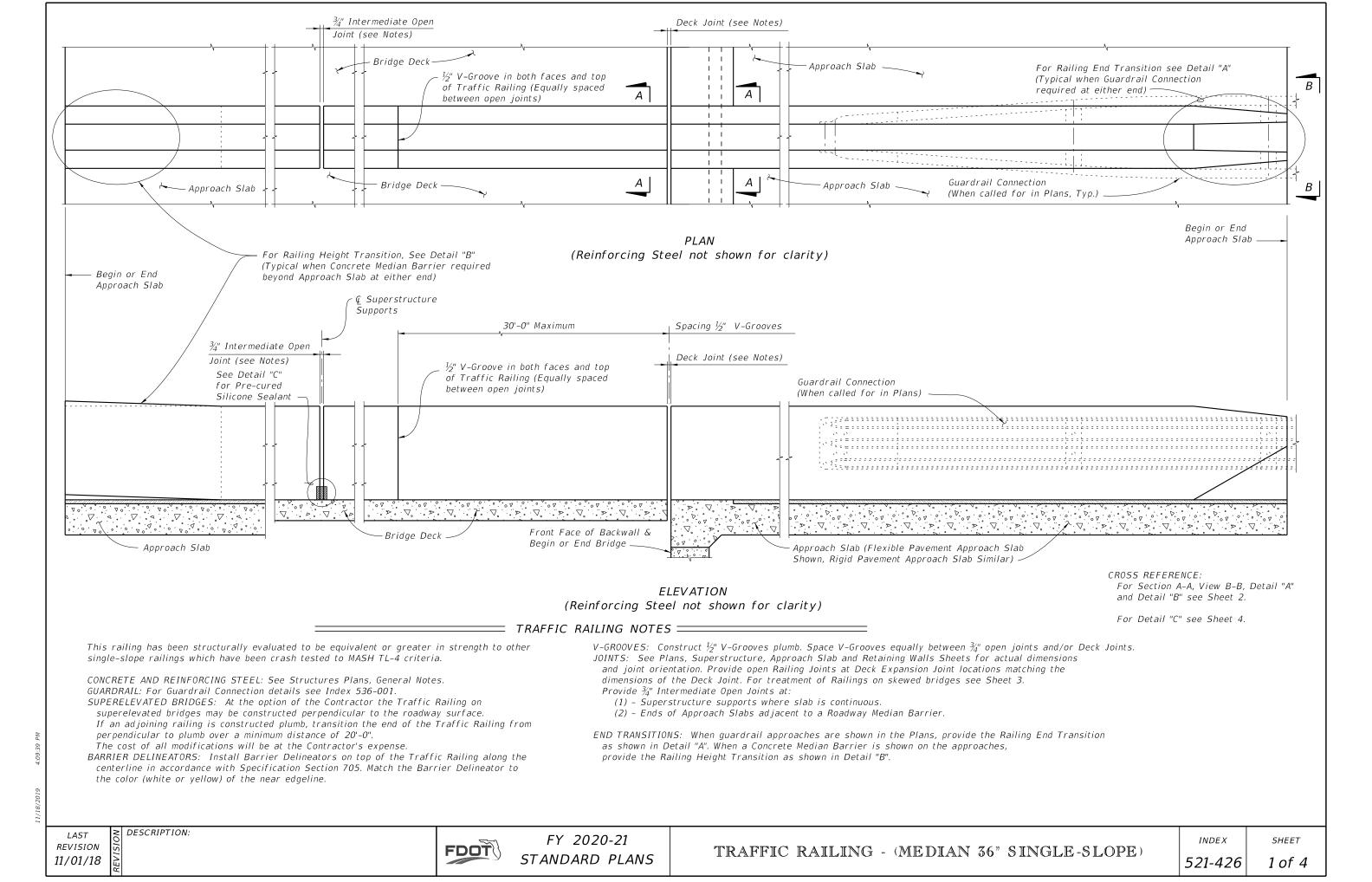


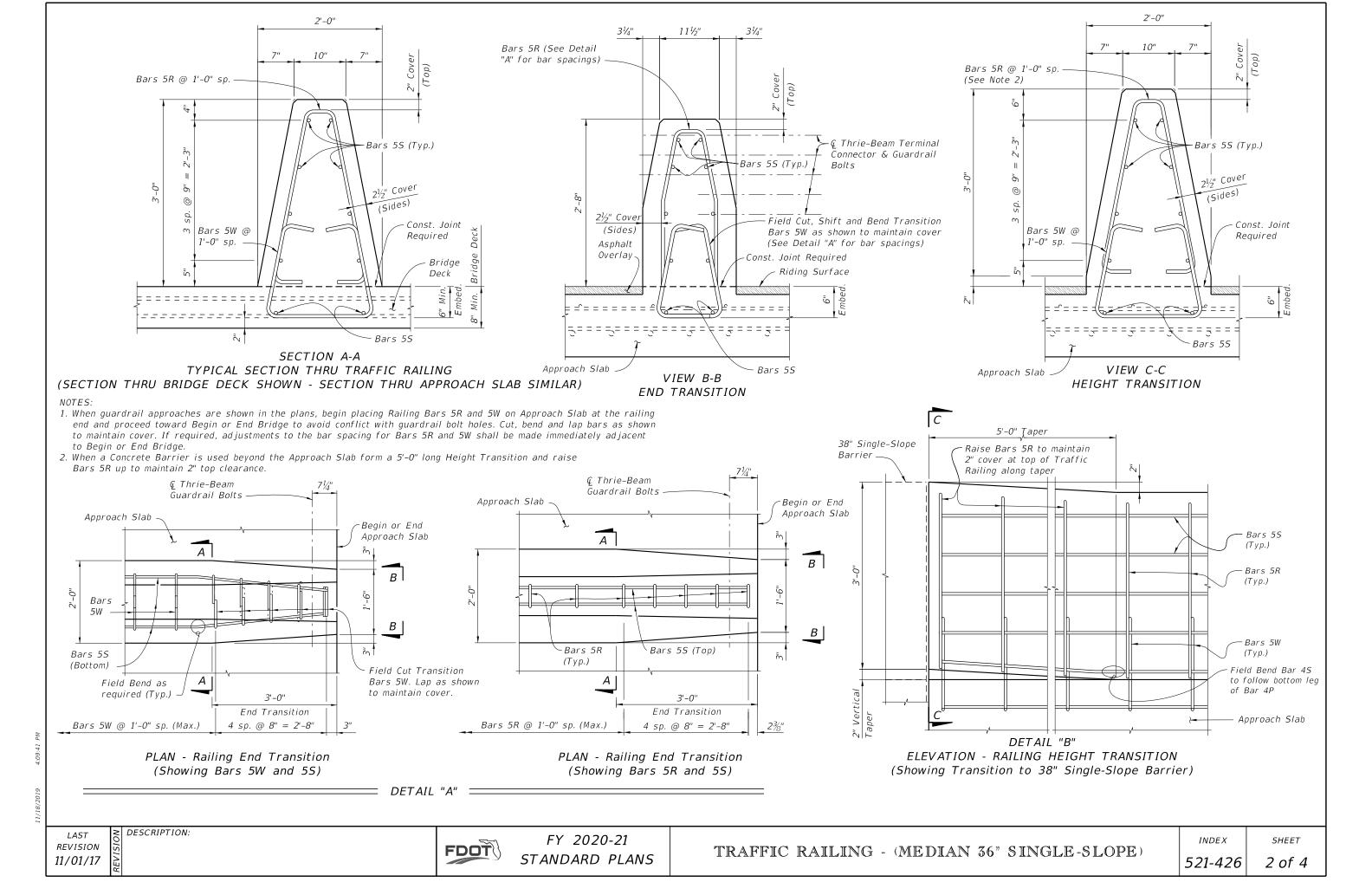
SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.095
Reinforcing Steel	LB/LF	25.90

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope.)





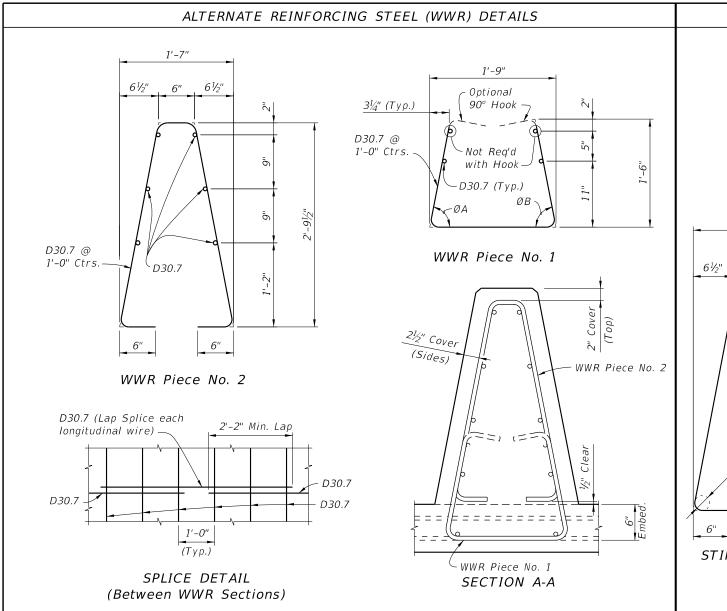


PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4) ¾" Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the Ç of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at & Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

DESCRIPTION:



ROADWAY ON SLOPE AT CROWN CROSS-SLOPE ØΑ ØВ ØΑ ØВ 7*9*° 7*9°* 79° 79° 0% to 2% >2% to 6% 81° 77° 79° 79° 79° 84° 74° 79° >6% to 10%

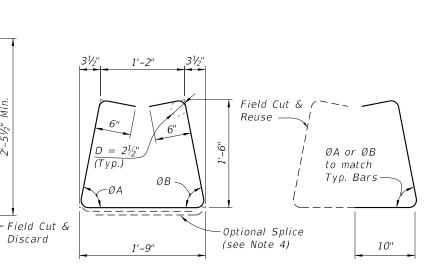
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

ØA and ØB shall be 79° if Contractor elects to place railing perpendicular to the deck, and approach slabs.

BILL OF	REINFORC	ING STEEL
MARK	SIZE	LENGTH
R	5	7'-2"
5	5	As Reqd.
W	5	5'-10"

Length as Required

BAR 5S



STIRRUP BAR 5R TRANSITION STIRRUP BAR 5R (5 required per Railing End Transition)

Field Bend as required

to maintain

cover

STIRRUP BAR 5W

TRANSITION STIRRUP BAR 5W To Be Field Cut (10 required per Railing End Transition)

REINFORCING STEEL NOTES:

6"

1'-7"

6"

 $D = 2\frac{1}{2}$ "

61/2"

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 4. At the Contractor's option, Bars 5W may be fabricated as a two piece bar with a 1'-2" lap splice of the bottom legs.

WELDED WIRE REINFORCEMENT NOTES:

DESCRIPTION:

- 1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 5R, 5S and 5W. WWR must meet the requirements of Specification Section 931.
- 2. WWR at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The bottom of Piece 1 shall be cut to allow overlap.
- 3. Place WWR panels so as to minimize the end overhang of longitudinal wires at Railing Ends and Open Joints. Overhangs greater than 6" are not permitted.

Pre-cured Silicone Sealant 4" wide (Typ.) (Typ.)

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.

ESTIMATED TRAFFIC RAILING QUANTITIES ITEM UNIT QUANTITY		
Reinforcing Steel	LB/LF	23.99

(The above quantities are based on a crowned roadway, with a 2% cross slope)

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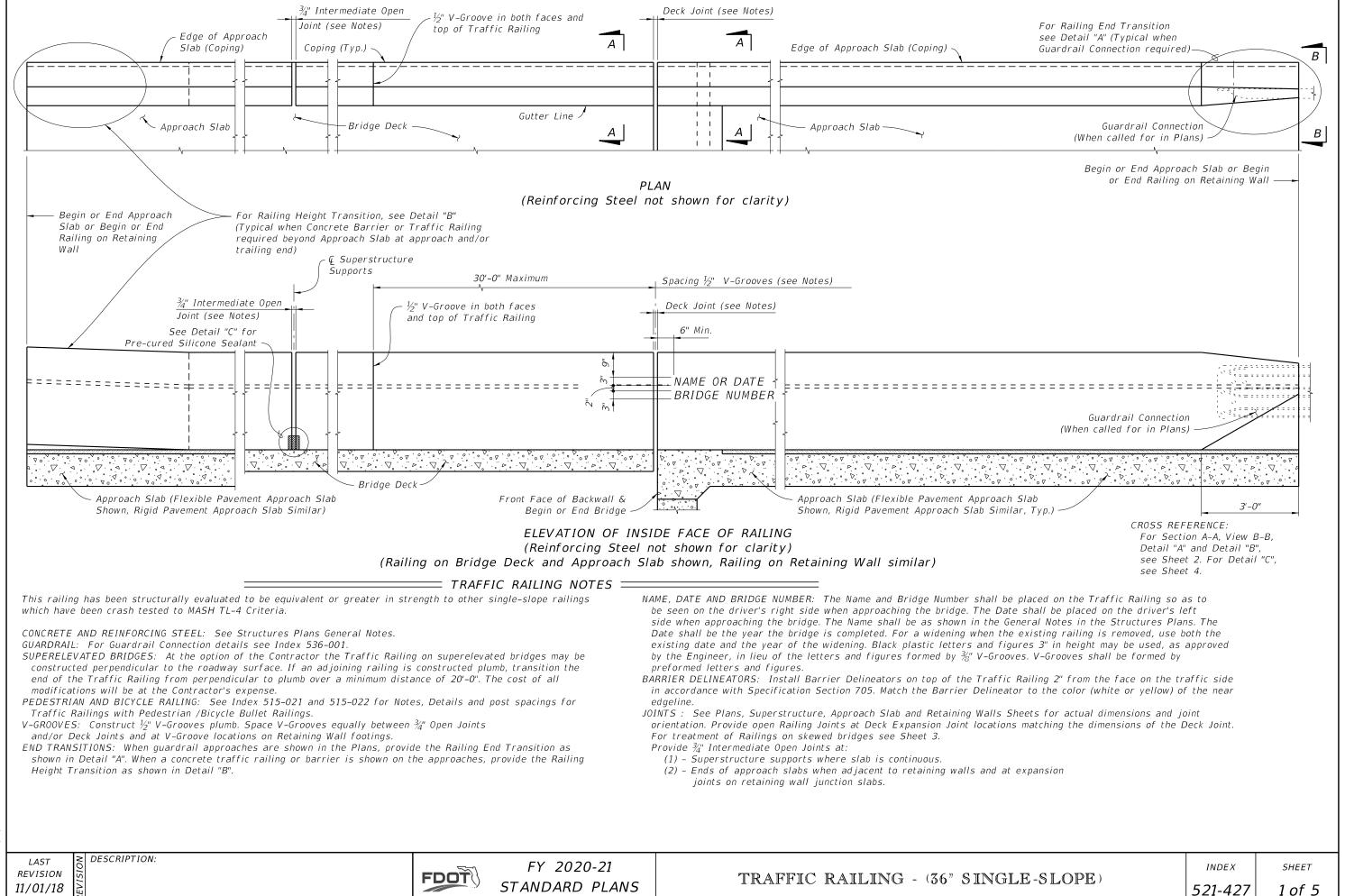
FY 2020-21 STANDARD PLANS

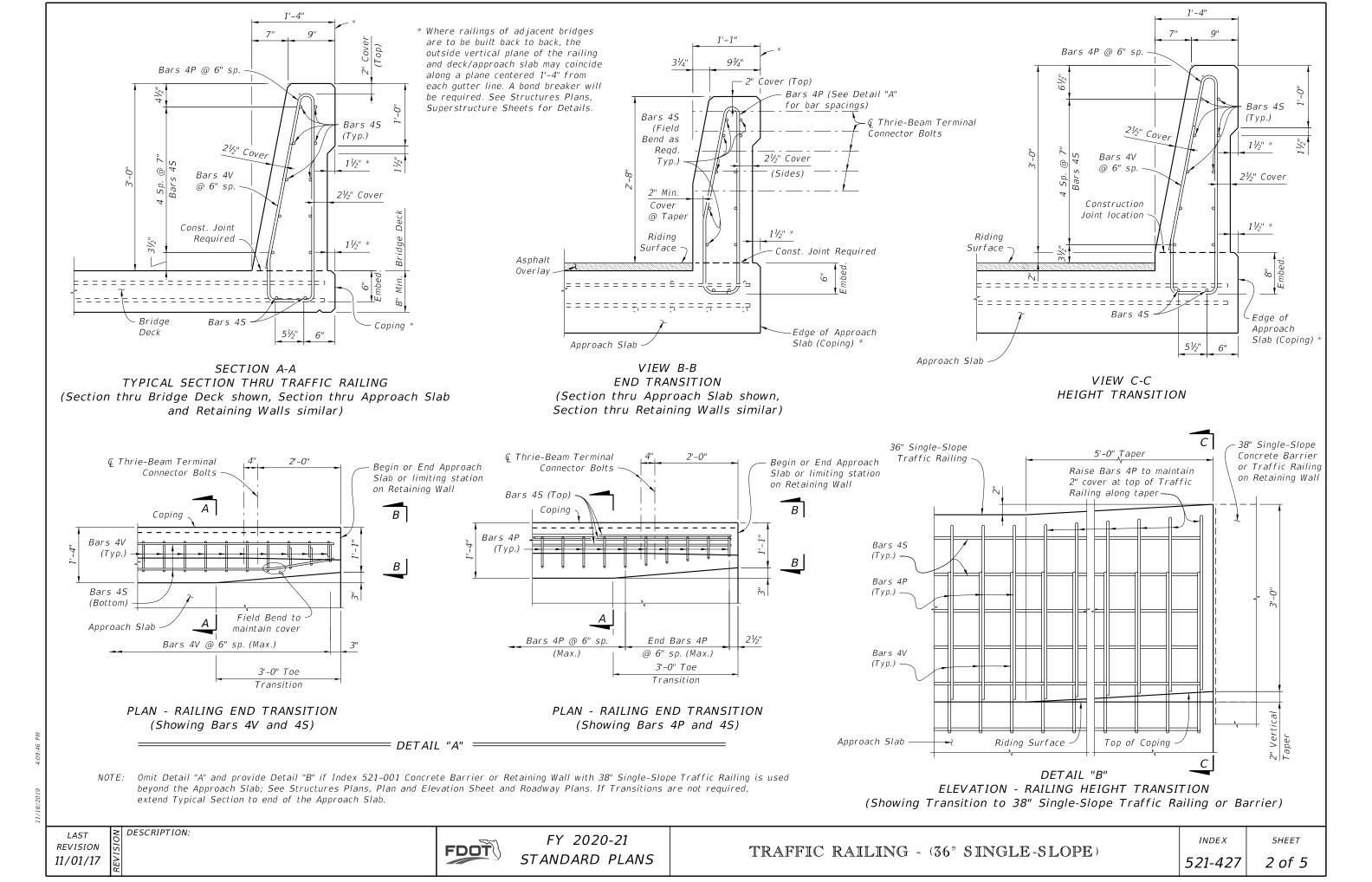
TRAFFIC RAILING - (MEDIAN 36" SINGLE-SLOPE)

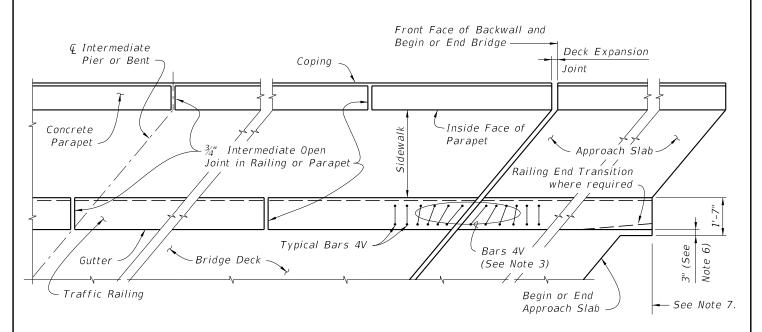
Discard

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SHEET



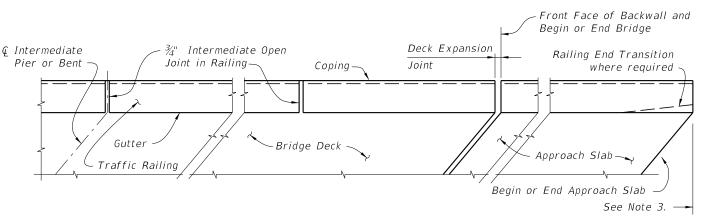




PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK, SINGLE-SLOPE TRAFFIC RAILING AND PEDESTRIAN/BICYCLE RAILING INDEX 521-820 or 521-825, OTHER TRAFFIC RAILINGS SIMILAR

NOTES:

- 1) Concrete Parapet reinforcement is not effected by skew angle, see Index 521-820 for details.
- 2) Parapet expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 3) Traffic Railing reinforcement vertical Bars 4V & 4P may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement. Bars 4V adjacent to expansion joints shall be field adjusted to maintain clearance and spacing, extra Bars 4V will be required. Cut bottom horizontal portion of 4V Bars to maintain maximum horizontal length to each vertical leg being placed. Discard the remainder of the bar. Rotate cut bars to maintain clearance.
- 4) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 5) ¾" Intermediate Open Joints and V-Grooves in railing and parapet shall be placed perpendicular or radial to the gutter line or inside face of parapet line. See Structures Plans, Superstructure Sheets for locations.
- 6) At begin or end approach slab extend slab at the railing ends 3" (gutter side or back face of railing as required) as shown to provide a base for casting of the railing. Field trim toe of Bars 4V by 1 inch as required to maintain concrete cover at edge of deck.
- 7) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.



PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SINGLE-SLOPE TRAFFIC RAILING, OTHER TRAFFIC RAILINGS SIMILAR

NOTES:

- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 2) ¾" Intermediate Open Joints and ½" V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 3) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent to Begin or End Bridge.

GENERAL NOTES:

- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes as applicable.
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing.
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

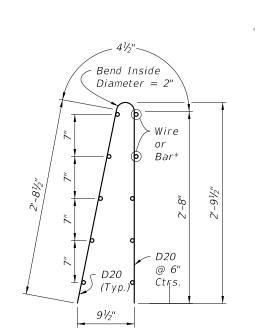
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LAST REVISION 11/01/17

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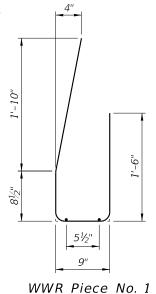
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FY 2020-21 STANDARD PLANS LOPE)

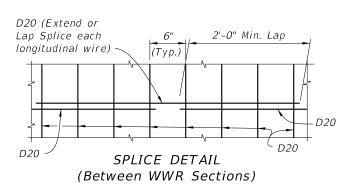


*Longitudinal D20 Wires or #4 Bars may be tied.

ALTERNATE REINFORCING STEEL (WWR) DETAILS



WWR Piece No. 2



WWR Piece No. 2 21/2" Cover 21/2" Cover ➤ WWR Piece No. 1

WELDED WIRE REINFORCEMENT NOTES:

- 1. At the option of the Contractor deformed Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 4P, 4S and 4V. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.
- 2. WWR at Railing End Transition shall be field bent inward as required (Piece 2) to maintain cover. The bottom of the vertical wires (D20) in Piece 2 shall be cut a maximum of 4 inches and the gutter side portion bent inward as required to allow placement.

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

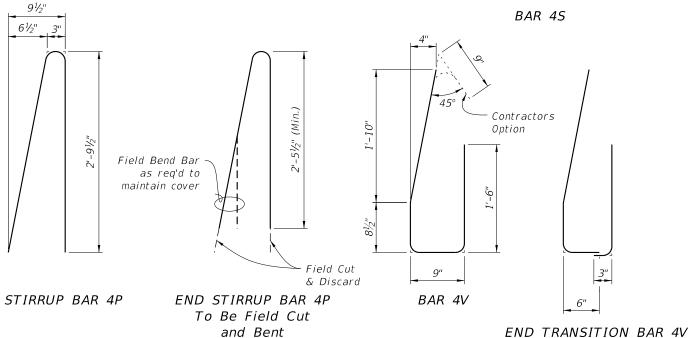
ROADWAY	LOW GUTTER	HIGH GUTTER
CROSS-SLOPE	ØВ	ØB
0% to 2%	90°	90°
2% to 6%	87°	93°
6% to 10%	84°	96°

ØB shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.

BILL OF REINFORCING STEEL			
MARK	LENGTH		
Р	4	5'-11"	
5	4	As Reqd.	
V	4	4'-10"	

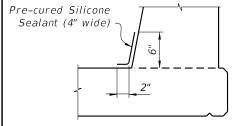
Length as Required





REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The $8lac{1}{2}$ " vertical dimensions shown for Bar 4V is based on a 6" embedment into the bridge deck without a raised sidewalk. If a raised sidewalk is to be provided, increase this dimension to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 2'-0".

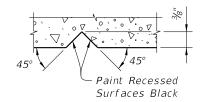


DESCRIPTION:

DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. Include the cost of the Pre-cured Silicone Sealant in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM	UNIT	QUANTITY		
Concrete	CY/LF	0.107		
Reinforcing Steel	LB/LF	24.78		

(The above quantities are based on a 2% deck cross slope; railing on low side of deck.)

REVISION 11/01/17

FDOT

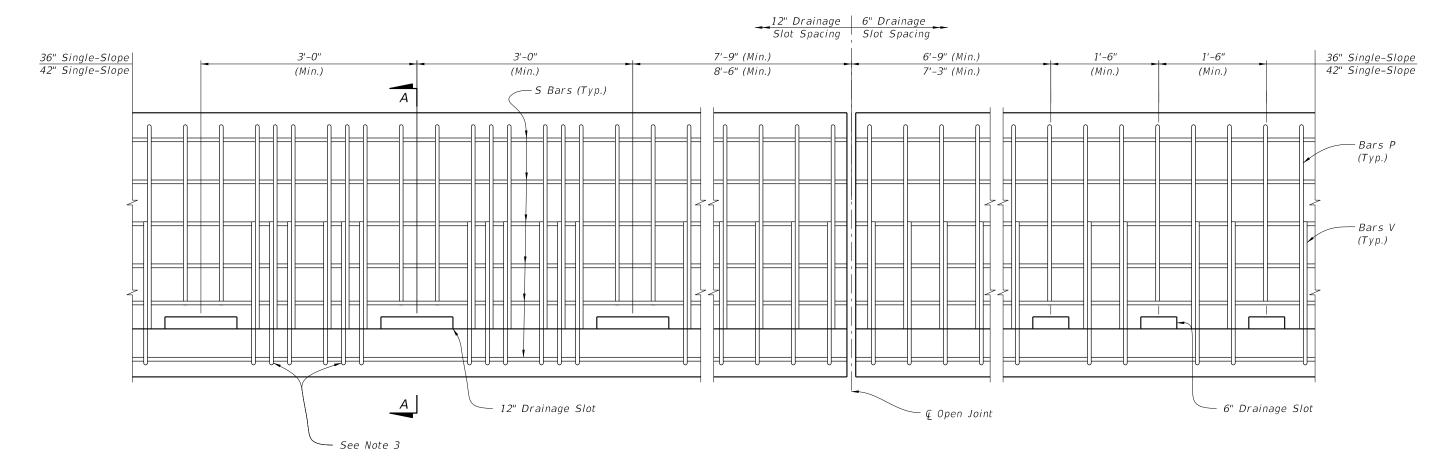
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TRAFFIC RAILING - (36" SINGLE-SLOPE)

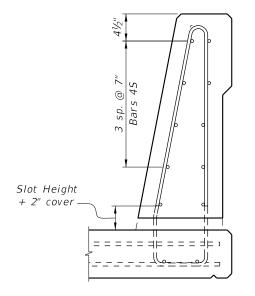
INDEX

Field Cut and Lapped

SHEET 4 of 5







SECTION A-A 36" Single-Slope Shown Other traffic railings similar

DRAINAGE SLOT NOTES:

- 1. Use only when required for safety. See Plans for locations and size of drainage slots.
- 2. Maintain 2" minimum cover to all reinforcing. Trim P Bars over drainage slots and raise bottom S bars as necessary to maintain cover.
- 3. For slots greater than 6" in length, add additional vertical bars (V & P) on each side of the opening.
- 4. Drainage slot heights are 2" or 3". See the plans for size and location details.

REVISION 11/01/19

DESCRIPTION:

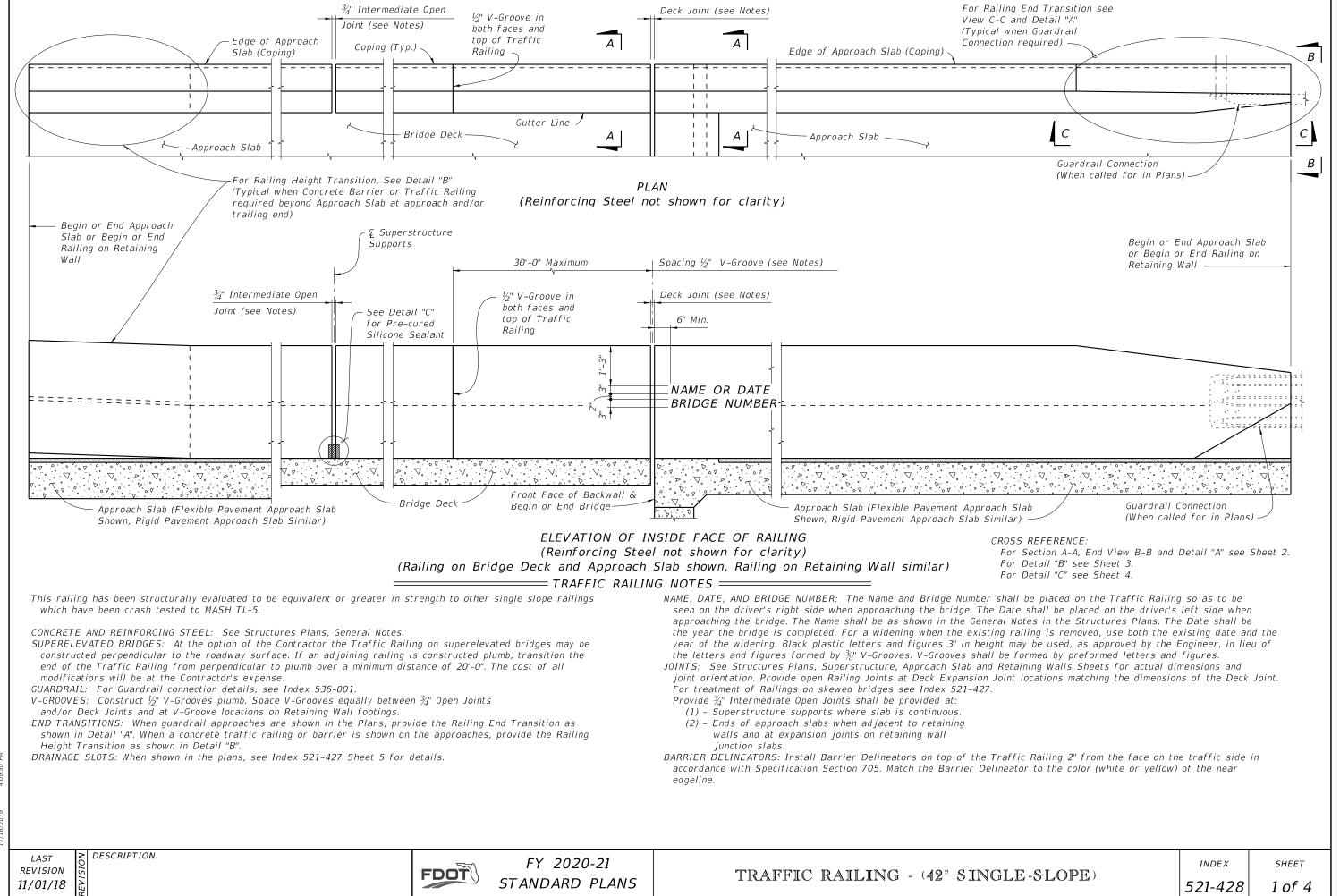
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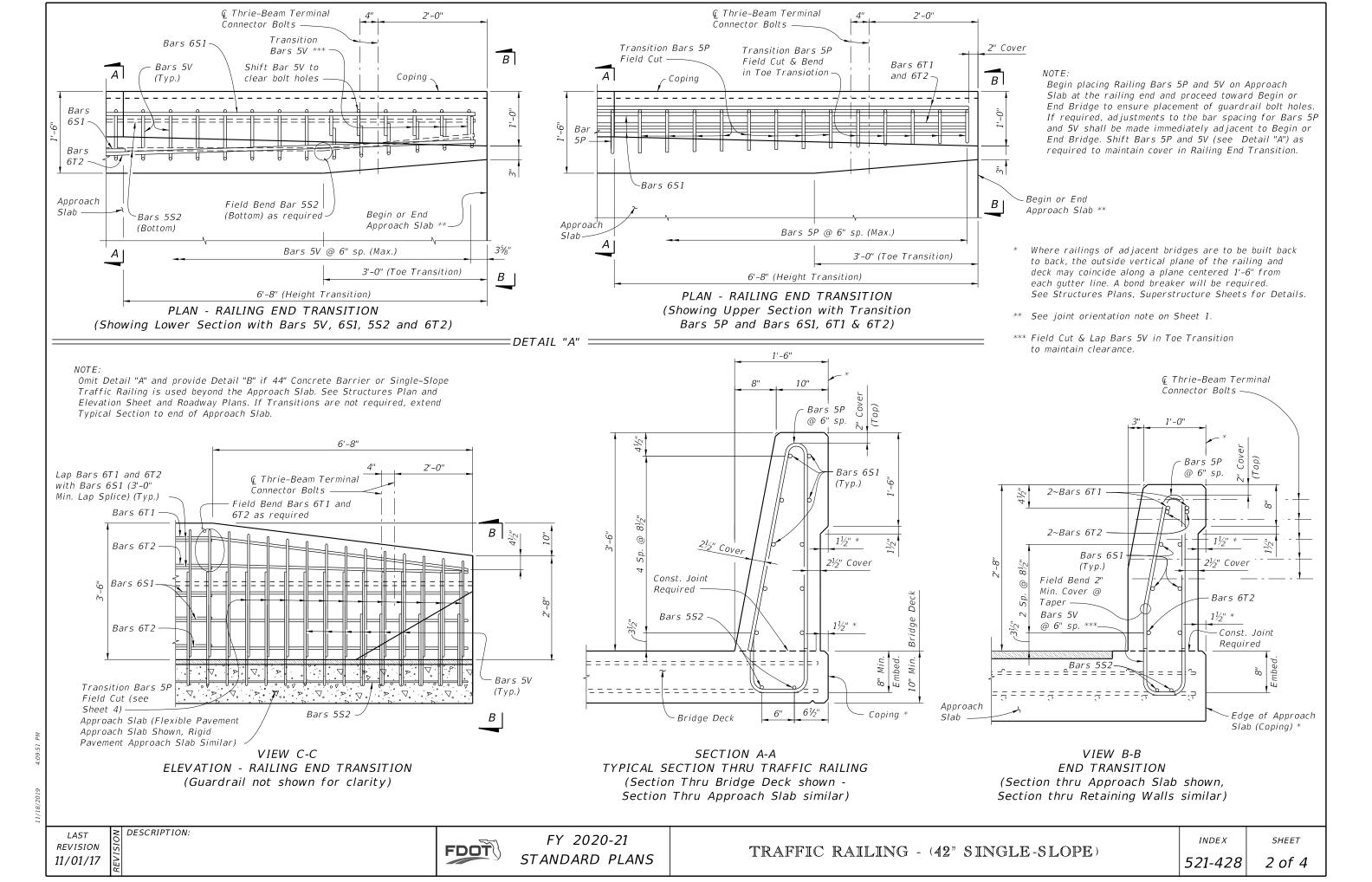
TRAFFIC RAILING - (36" SINGLE-SLOPE)

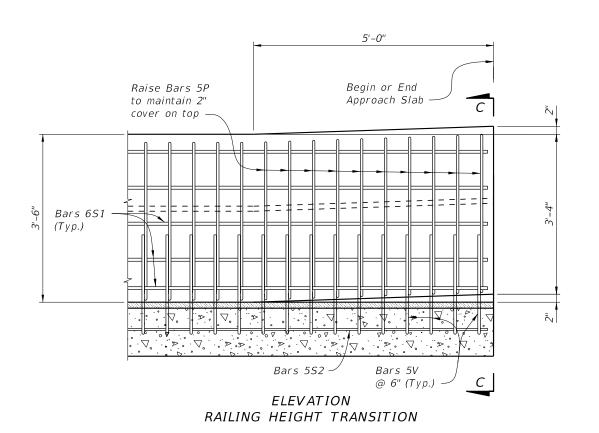
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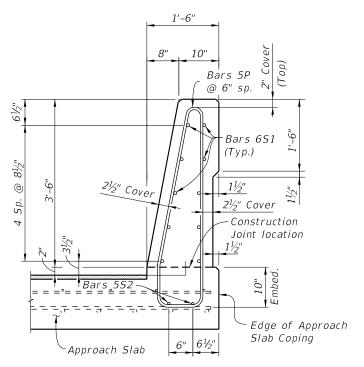
SHEET 5 of 5



11/18/20



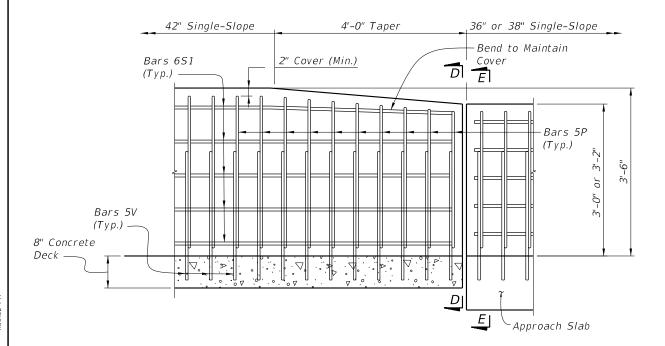


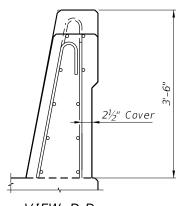


VIEW C-C RAILING HEIGHT TRANSITION

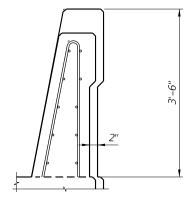
- 1. Provide Detail "B" height transition where 42" Single-Slope Traffic Railings increase to 44" Barriers beyond flexible pavement approaches.
- 2. Work Detail "B" with Index 400-090.
- 3. Provide Detail "C" height transition where 42" Traffic Railings are required on bridge, and 36" or 38" Barriers are shown on approaches.
- 4. Work Detail "C" with Indexes 400-090 or 400-091, 521-427, and 521-610 as necessary.
- 5. Field cut 5P Bars as shown to maintain 2" min. (4" max.) cover at top of traffic railing.

DETAIL "B" =





VIEW D-D RAILING HEIGHT TRANSITION (Begin/End of Bridge) (Bars 5V not shown for clarity)



SECTION E-E (Index 400-091 Shown, 400-090 Similar) (Index 521-427 Bars 4V not shown for Clarity)

DETAIL "C" =

REVISION 11/01/19

DESCRIPTION:

FDOT

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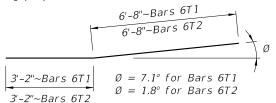
SHEET 3 of 4

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
Р	5	7'-0"		
S1	6	As Reqd.		
52	5	As Reqd.		
T1 & T2	6	10'-0"		
V	5	5'-9"		

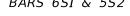
ROADWAY CROSS-SLOPE	LOW GUTTER	HIGH GUTTER
	ØВ	ØВ
0% to 2%	101°	101°
2% to 6%	98°	104°
6% to 10%	95°	107°

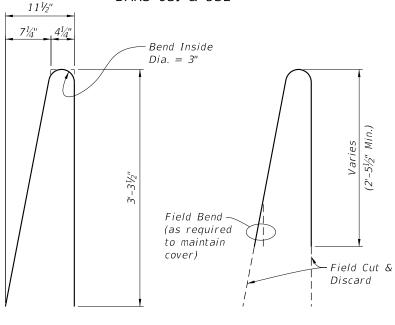
ØA and ØB shall be 90° if Contractor elects to place Railing perpendicular to the Deck.

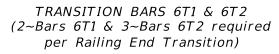


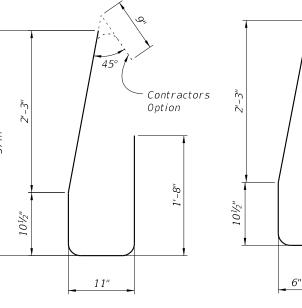
Length as Required

BARS 651 & 552









STIRRUP BAR 5P

TRANSITION STIRRUP BAR 5P To Be Field Cut (10 of each required per Railing End Transition)

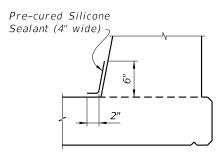
STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut and Lapped

REINFORCING STEEL NOTES:

DESCRIPTION:

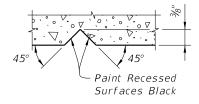
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 6S1 may be continuous or spliced at the construction joints. Lap splices for Bars 6S1 and 5S2 shall be a minimum of 3'-0" and 2'-2", respectively.
- 4. The Contractor may utilize deformed WWR when approved by the Engineer. WWR must meet the requirements of Specification Section 931.



DETAIL "C" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM	UNIT	QUANTITY		
Concrete	CY/LF	0.143		
Reinforcing Steel	LB/LF	39.34		

The estimated railing quantities are based on a 2% deck cross slope; railing on low side of deck.



Details for bridges on horizontally curved alignments are similar.

elevation markers are removed.

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

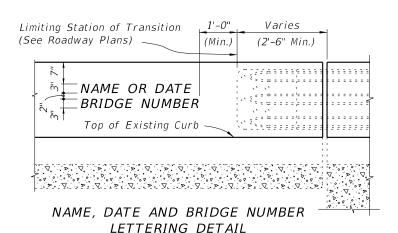
EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment). BRIDGES ON CURVED ALIGNMENTS: The details presented in these Indexes are shown for bridges on tangent alignments.

NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or $\frac{3}{6}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures. ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing

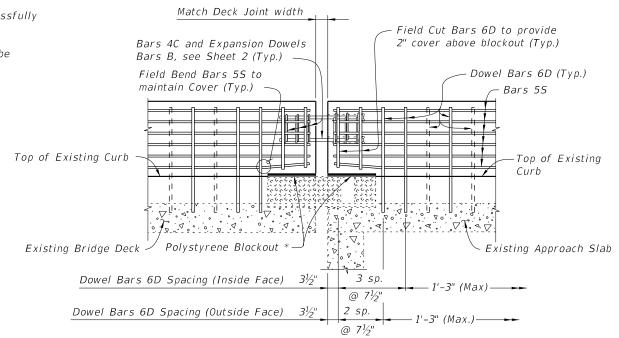
BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing 2" from the face on the traffic side in accordance with Specification Section 705. Match the Barrier Delineator color (white or yellow) to the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) includes all materials and labor required to construct the railing and incidental work as required for transition blocks, curbs, spread footing approaches, and Barrier Delineators.



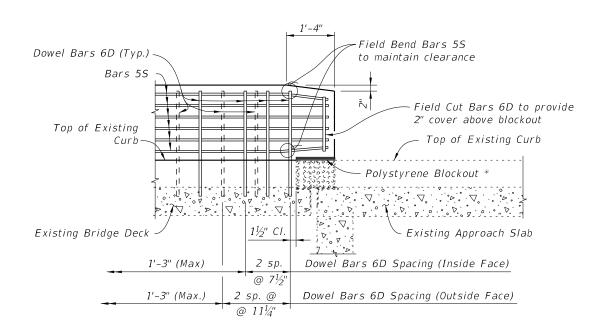
ESTIMATED TRAFFIC RAILING QUANTITIES				
UNIT	QUANTITY			
	9" Curb	Increment		
CY/FT	0.064	0.003 per in. height		
LB/FT	13.27	0.10 per in. length		
	UNIT CY/FT	UNIT QUAN 9" Curb CY/FT 0.064		

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index 521-484, Sheet 4 for Spread Footing Approach Quantities.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT - SCHEMES 2 THRU 5 (Begin or End Bridge Shown, Intermediate Joints Similar)

* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING FINGER/SLIDING PLATE JOINT AT BEGIN OR END BRIDGE - SCHEME 1 (Guardrail Transition not shown for clarity)

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FDOT

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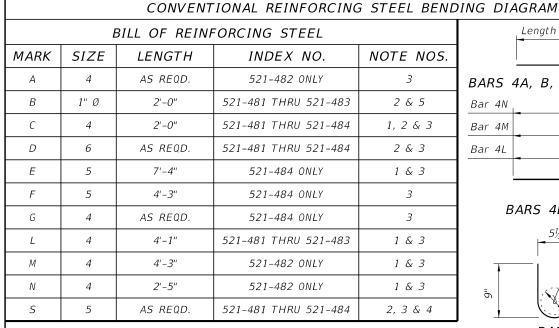
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) TYPICAL DETAILS & NOTES

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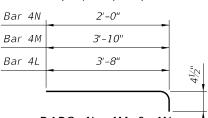
1 of 2

DESCRIPTION:



Length as Required

BARS 4A, B, 6D, 5F, 4G & 5S

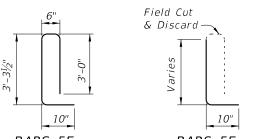


BARS 4L, 4M & 4N $D = 4\frac{1}{2}$ " Pin

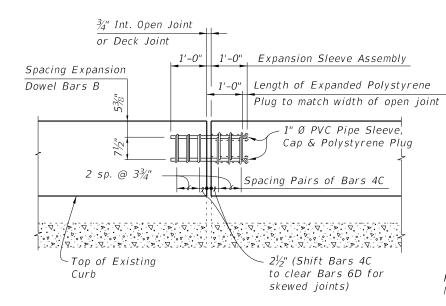
BARS 4C (12 required per open joint)

REINFORCING STEEL NOTES:

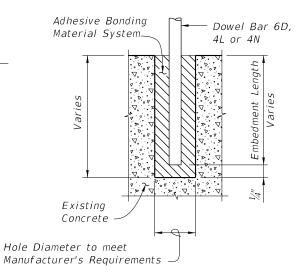
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.



BARS 5E BARS 5E (Typical Section) (Tapered End Transition)



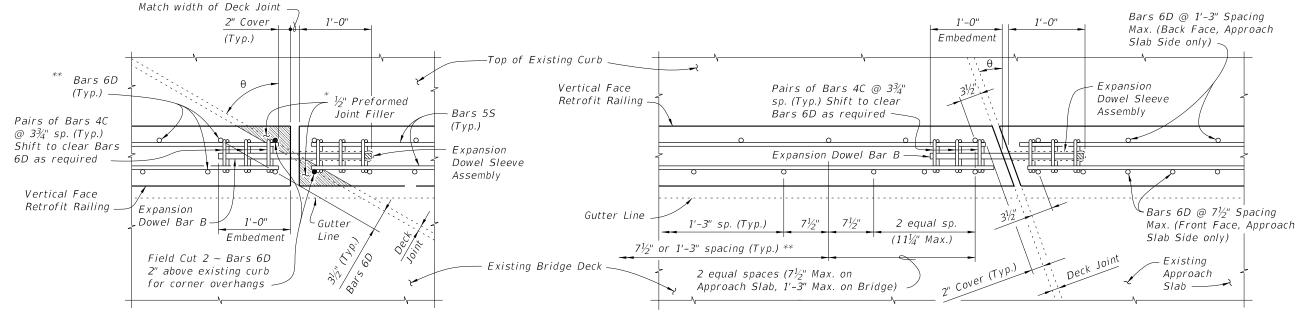
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



DOWEL DETAIL

Dowel Installation Notes:

- 1. Shift dowel holes to clear if the existing reinforcement is encountered.
- 2. See Index 521-481 thru 521-484 for required embedment length of Bars 6D, 4L or 4N.
- * $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.
- ** See Index 521-481 thru 521-484 for spacing of Bars 6D.



PARTIAL PLAN OF RAILING (SKEW ANGLE \theta GREATER THAN 20°) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

PARTIAL PLAN OF RAILING (SKEW ANGLE $\theta = 20^{\circ}$ OR LESS) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

SKEW DETAIL

REVISION 11/01/16

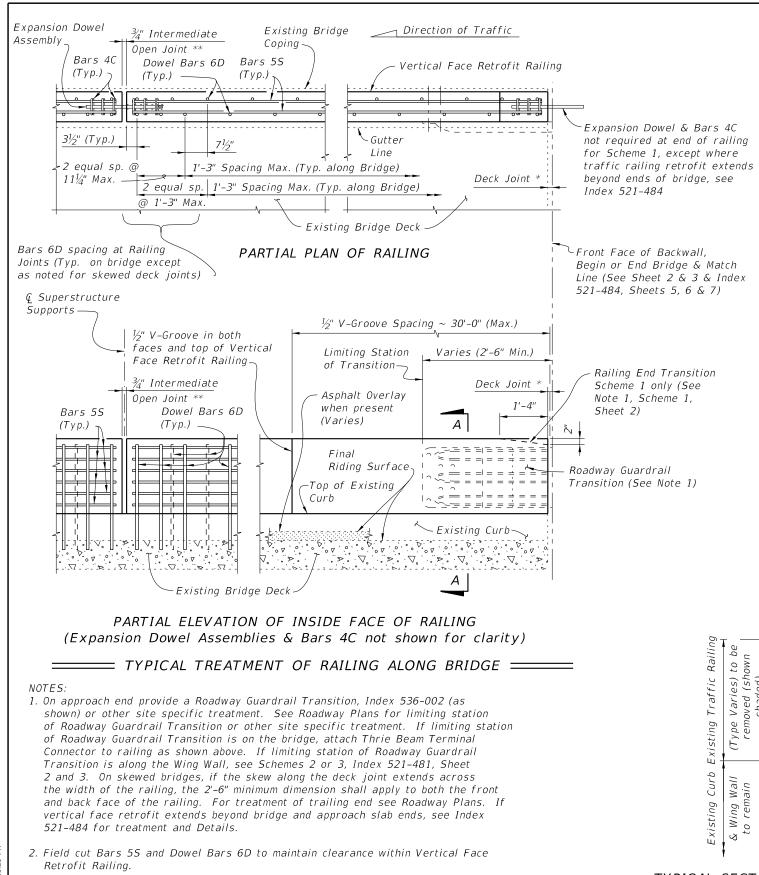
DESCRIPTION:

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TRAFFIC RAILING - (VERTICAL FACE RETROFIT) TYPICAL DETAILS & NOTES

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3. Where existing structure has been removed and not encased in new concrete; match

adjoining areas and finish flat by grouting or grinding as required. Exposed existing

reinforcing steel not encased in new concrete shall be burned off 1" below existing

TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Varies Skew Detail, Index 521-480. Provide open Railing Joints at Deck Expansion Joint locations matching the dimension of Varies Varies (3" max. and Preferred, 10" | (1" Min.) 1" Min., constant for full length of Retrofit) ← Thrie-Beam
 ← Thrie-Be (1) - Superstructure supports where slab is continuous. Guardrail Bolts *** Curb heights vary from 5" Min. to 1'-2" Max. 2" Cover (Typ.)Bars 55 (Typ.)Dowel Bars 6D -Existing Asphalt Overlay Curb when present 6" Min. Embed Min. (Varies) _

SECTION A-A TYPICAL SECTION THRU RAILING ON CURB WITH CORBELS

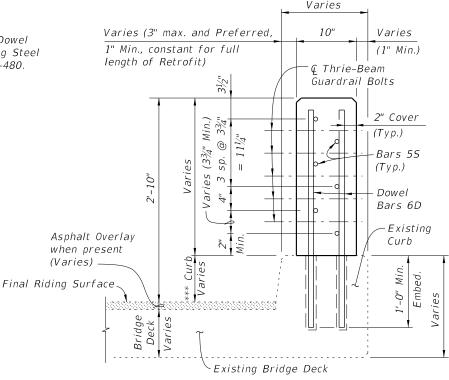
Existing Bridge Deck

CROSS REFERENCE:

the Deck Joint.

** Provide 3/4" Intermediate Open Joints at:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagrams see Index 521-480.



SECTION A-A TYPICAL SECTION THRU RAILING ON FULL DEPTH CURB (BRIDGE SHOWN, WING WALL SIMILAR)

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concrete and grouted over.

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) NARROW CURB

Final Riding Surface

Bridg

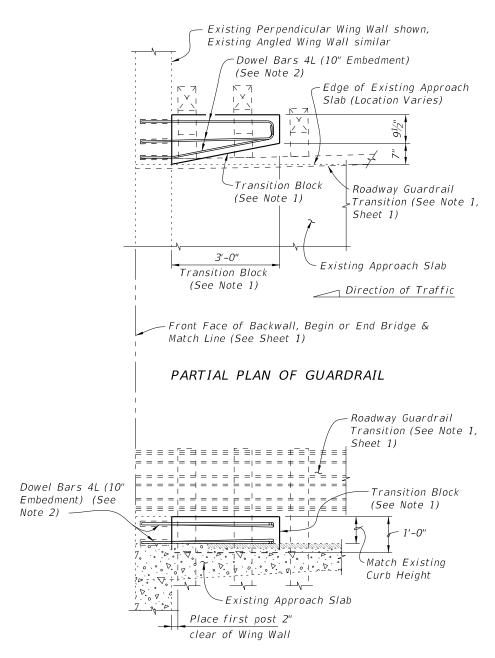
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REVISION

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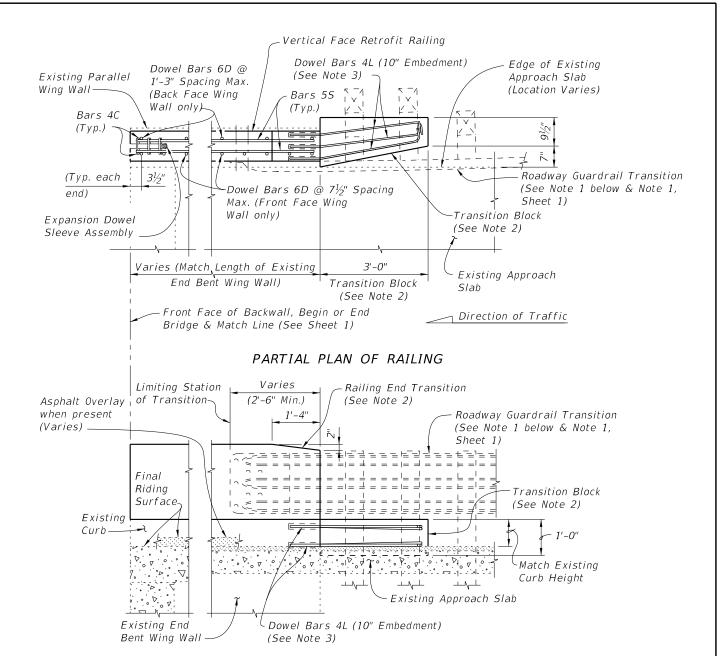
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

== SCHEME 1 ====== RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

DESCRIPTION:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

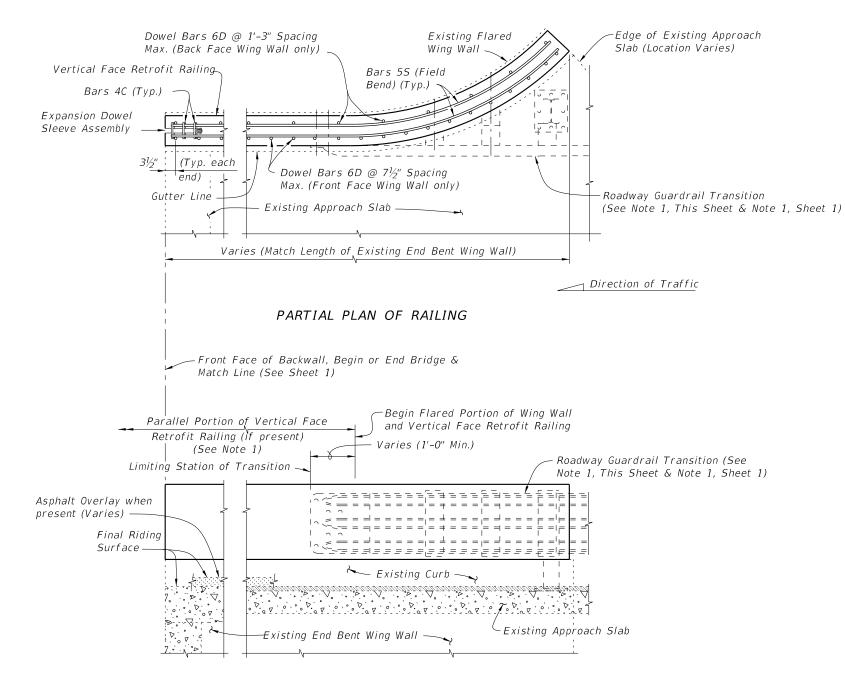


PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

____ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Index 521-481, Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING
(Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

SCHEME 3 RAILING END TREATMENT FOR
FLARED WING WALLS

SCHEME 3 NOTE:

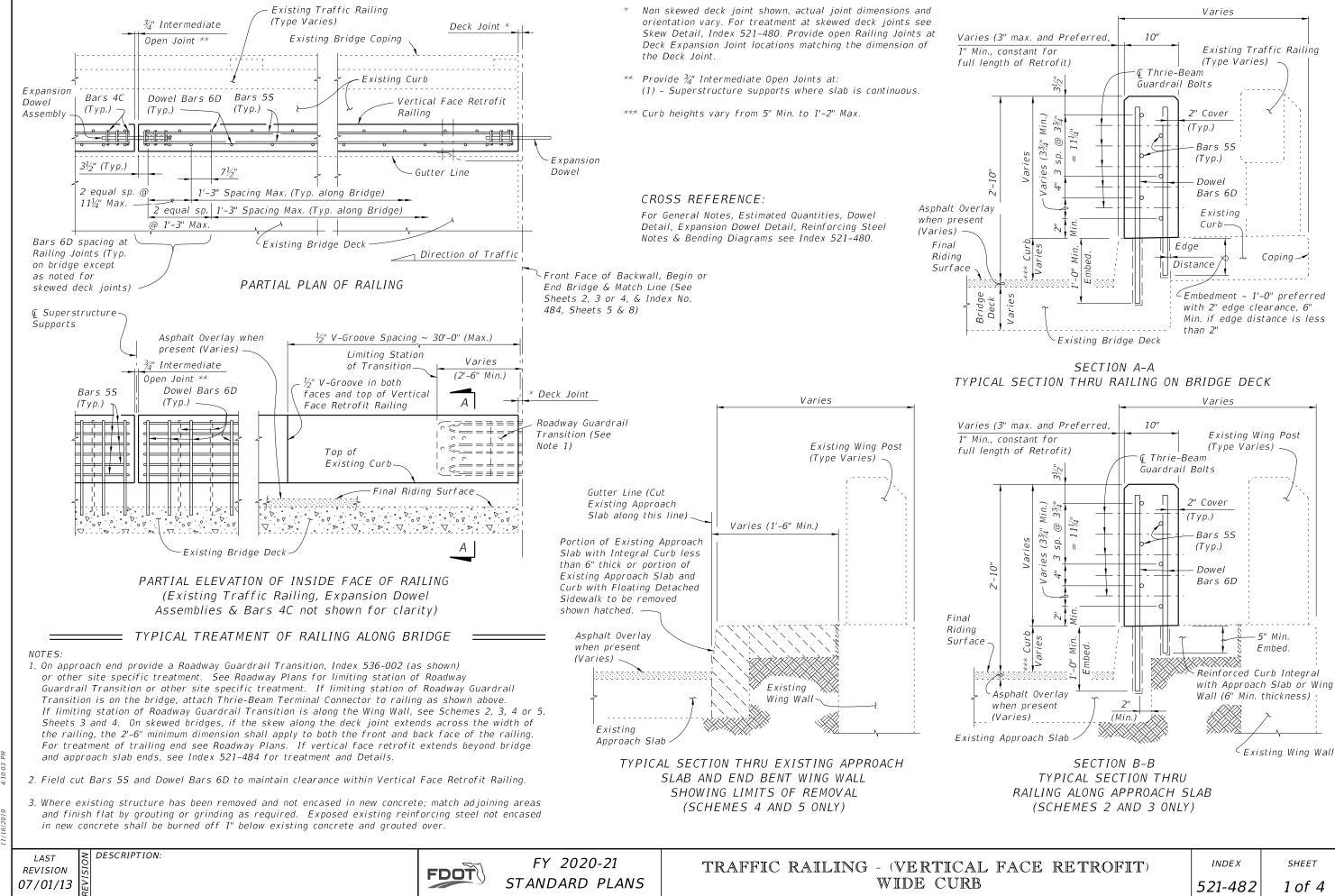
1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see

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11/18/2019

LAST DESCRIPTION:
REVISION 157
07/01/07

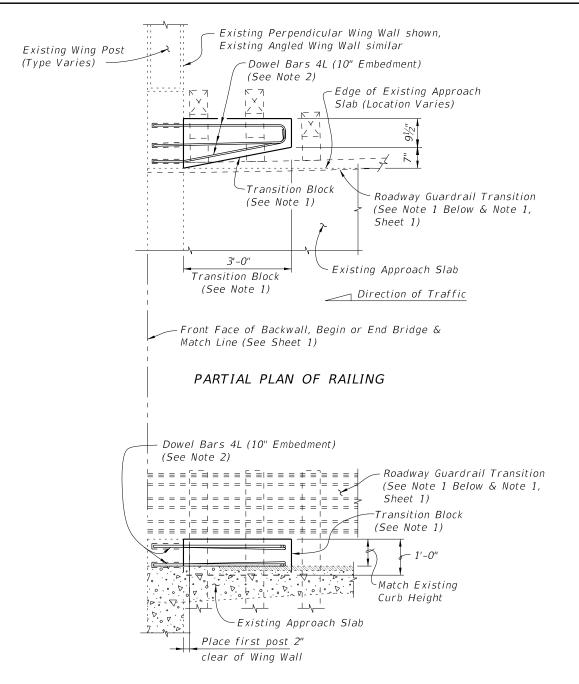








DESCRIPTION:



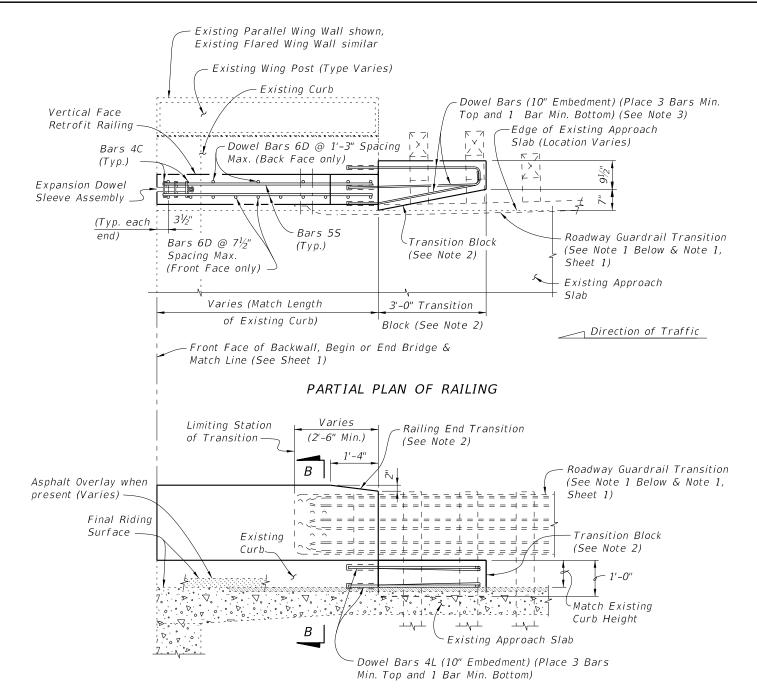
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

RAILING END TREATMENT FOR

PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.

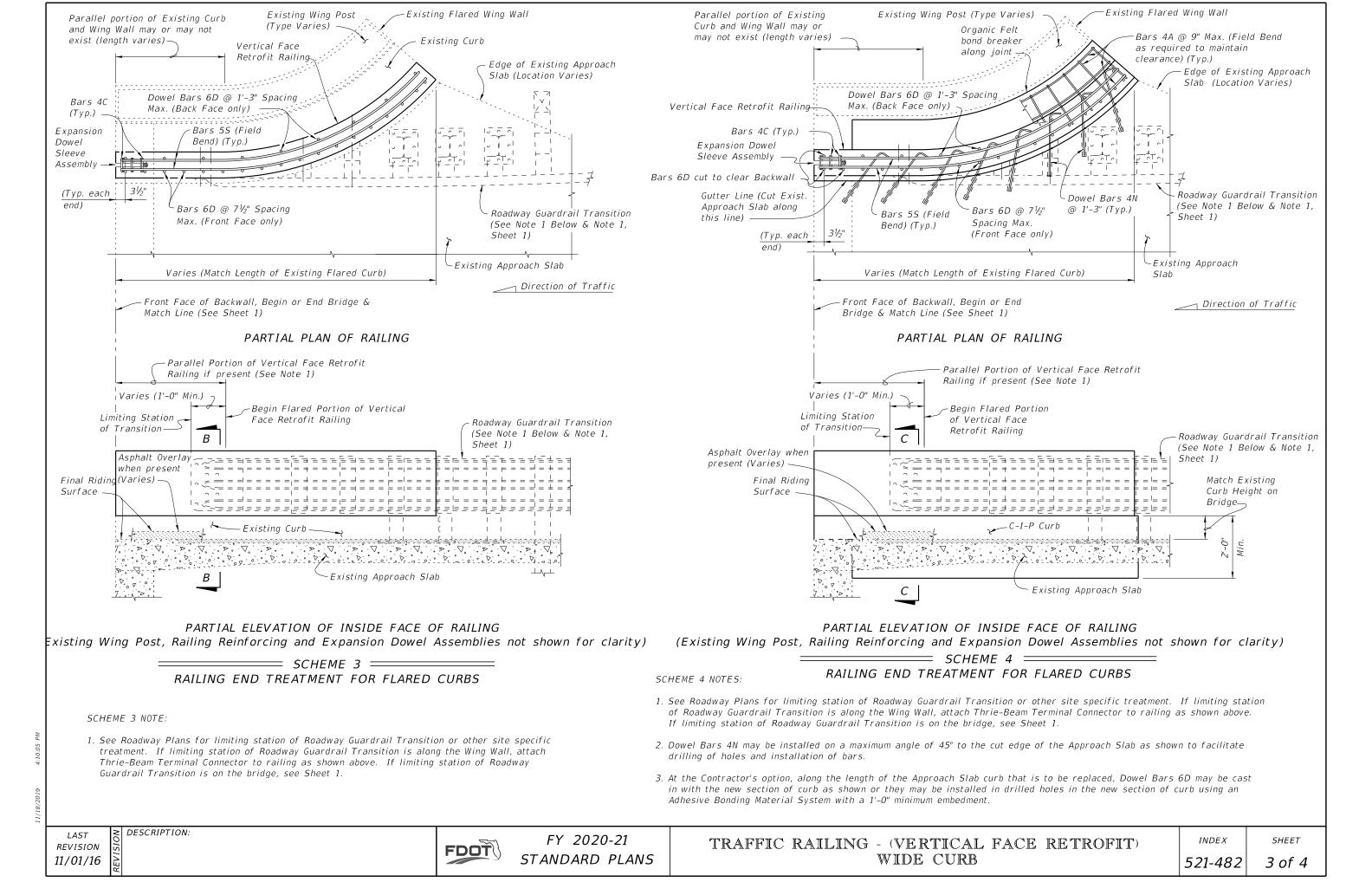


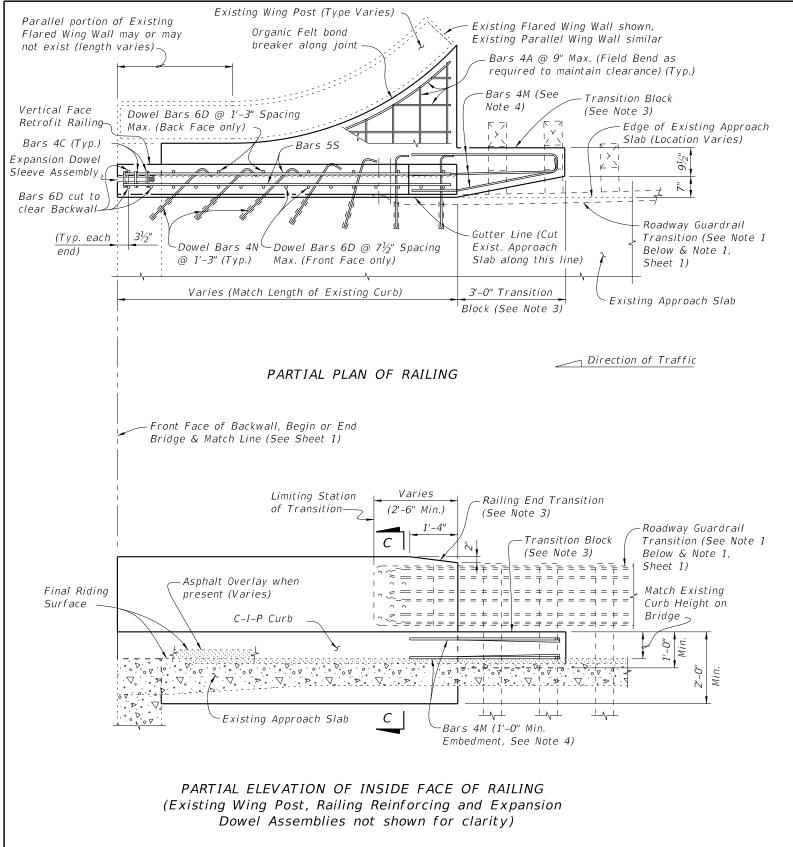
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.





Varies *** Curb heights vary from 5" Min. to 10" Max. Match height and shape of existing curb on bridge. Varies (1'-6" Min.) Varies (3" Preferred, 10" 1" Min., constant for full length of Retrofit) Existing Wing Post ← Thrie-Beam (Type Varies) Guardrail Bolts 2" Clear (Typ.)@ ; (3¾" sp. @ Bars 5S (Typ.)Dowel Bars 6D (See Note 5) Organic Felt bond breaker along joint Final Riding Surface Bars 4A @ 9" Max Asphalt Overlay when present 2" Min. Clear. Top (Varies) and Sides, 4" Min. Clear. Bottom -Existing Approach Slab Dowel Bars 4N @ 1'-3" (Typ.) Bars 4A @ 9" Max., Min. 3 full length bars required Top & Bottom (Field Bend to 10" Min. Embedment

SECTION C-C TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

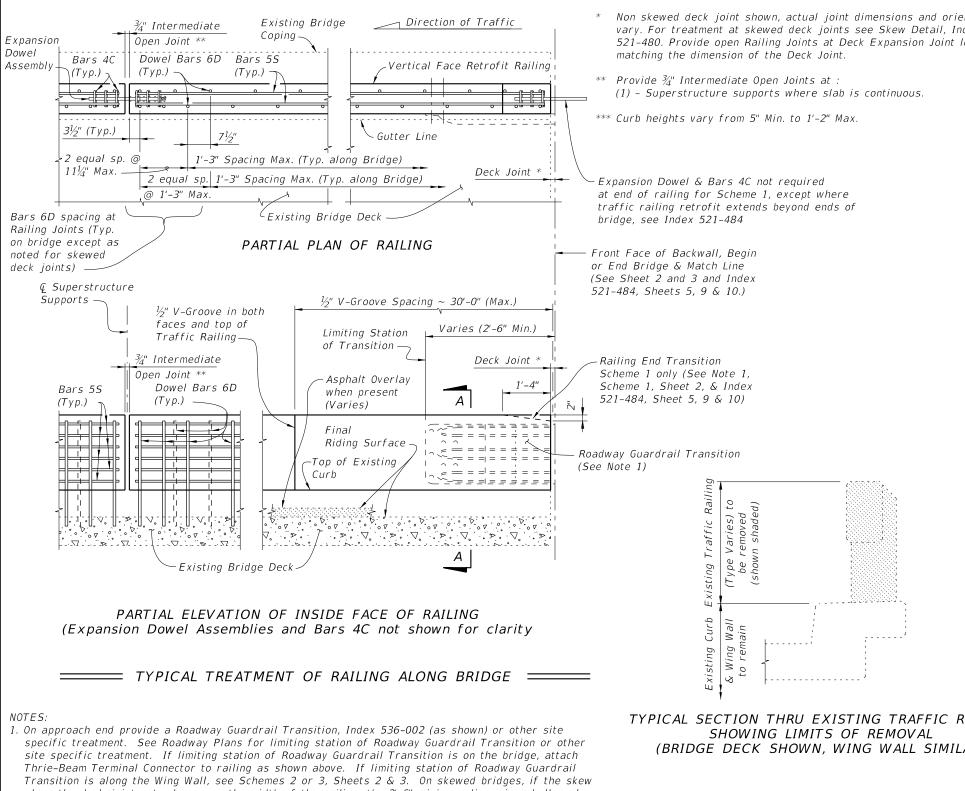
clear) (Typ.)

SCHEME 5 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.

______ SCHEME 5 _____

RAILING END TREATMENT FOR PARALLEL CURBS



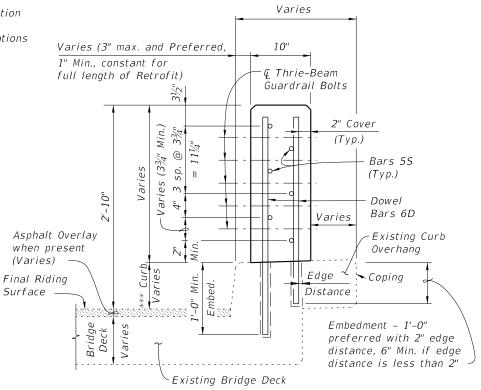
- along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing. For treatment of trailing end see Roadway Plans. If vertical face retrofit extends beyond bridge and approach slab ends, see Index 521-484 for treatment and Details.
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.
- 3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off 1" below existing concrete and grouted over.

Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index 521-480. Provide open Railing Joints at Deck Expansion Joint locations

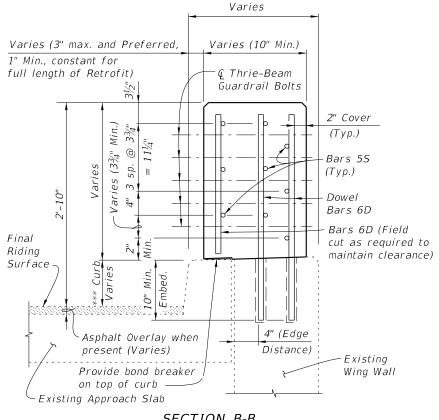
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagram see Index 521-480.



SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK



SECTION B-B TYPICAL SECTION THRU RAILING ON WING WALL

REVISION 07/01/13

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) INTERMEDIATE CURB

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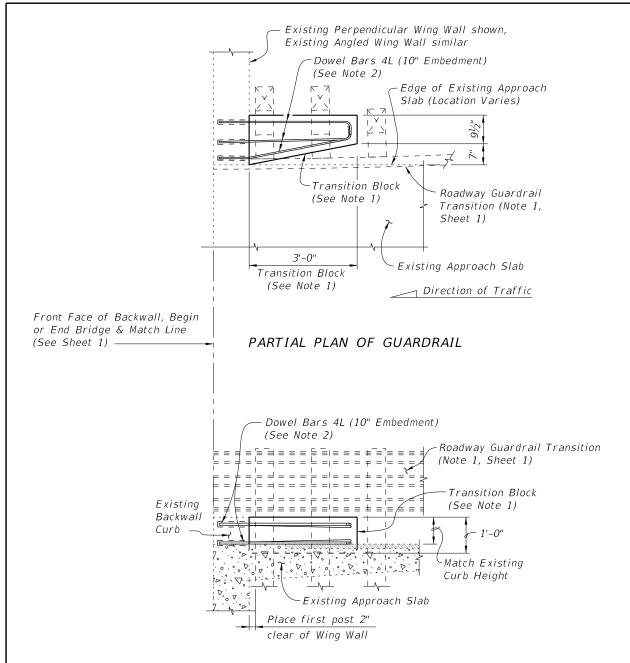
521-483





REVISION

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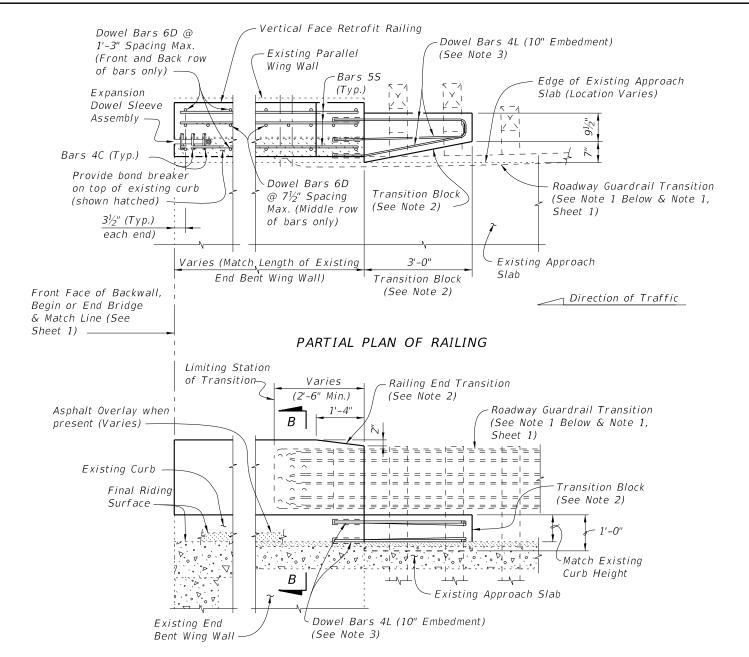
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

______ SCHEME 1 _____ RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

DESCRIPTION:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

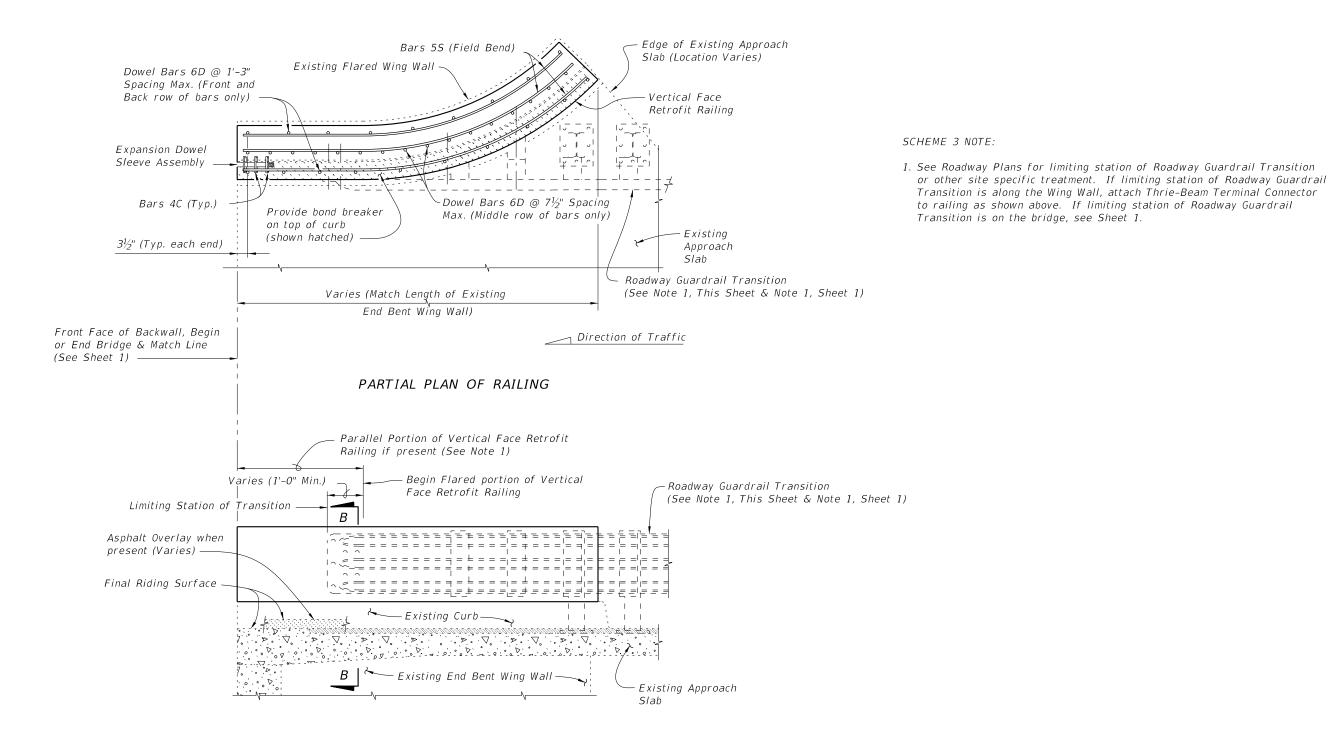
______ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.

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FY 2020-21 STANDARD PLANS



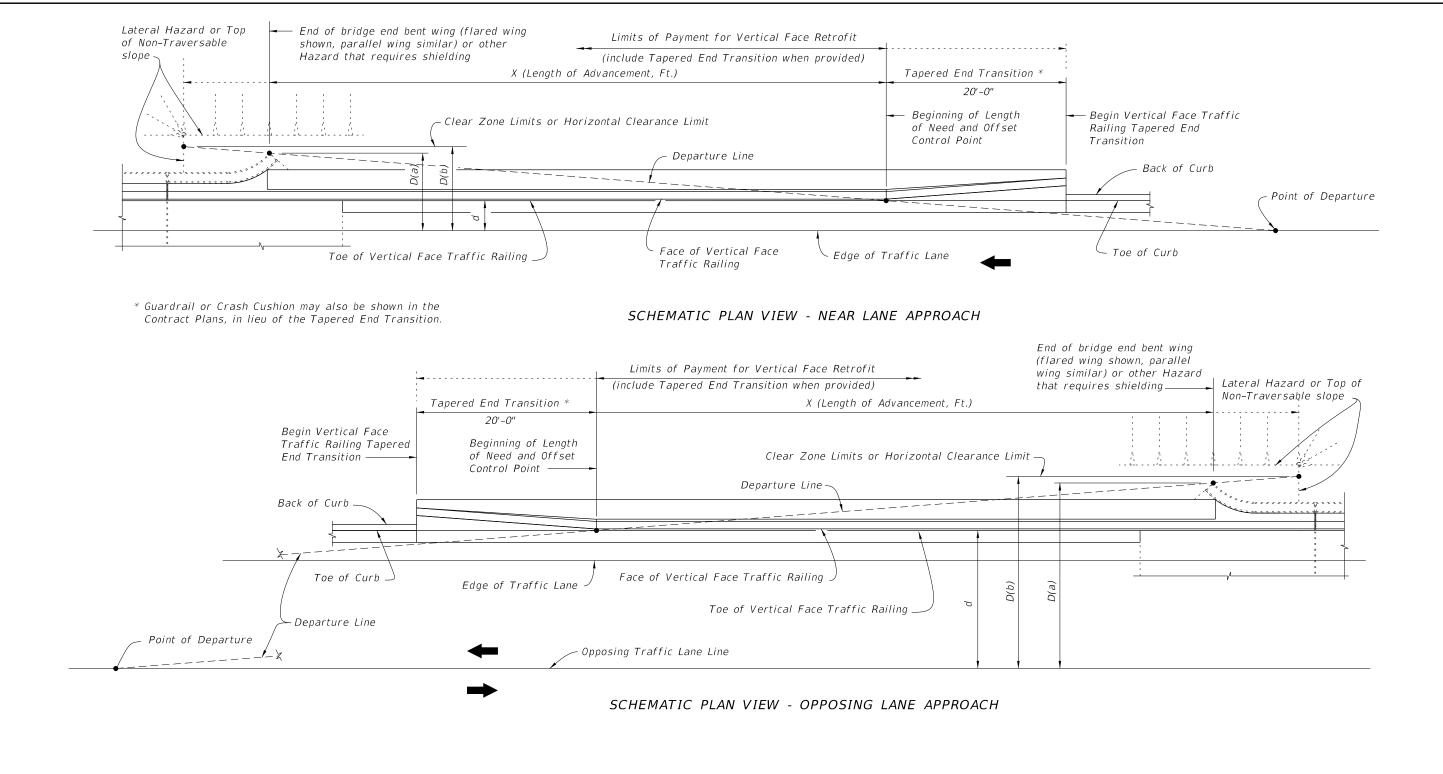
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

RAILING END TREATMENT FOR

FLARED WING WALLS

11/18/2019

LAST DESCRIPTION:
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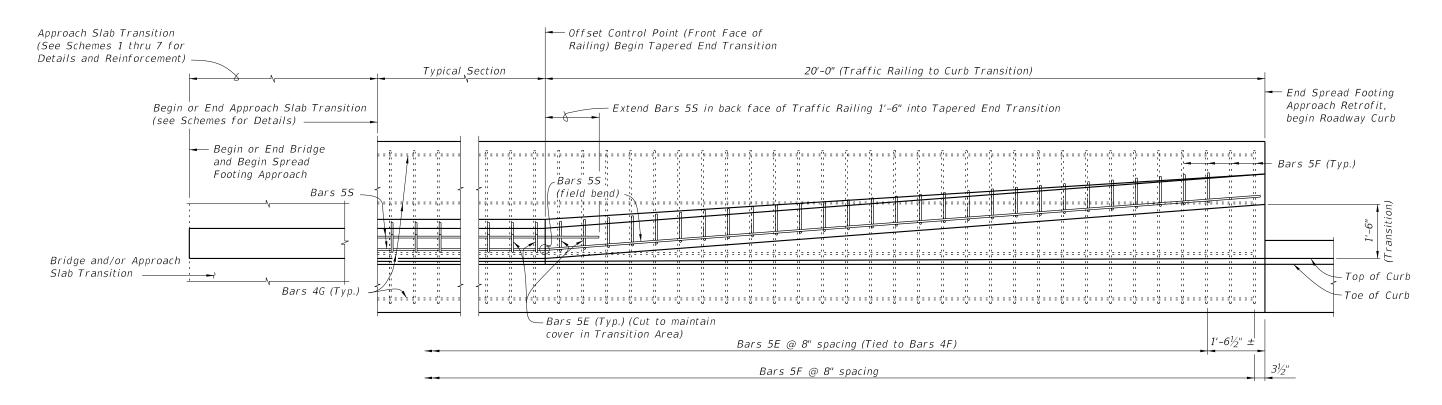
CROSS REFERENCES:

For General Notes, Dowel Details, Expansion Dowel Details, Reinforcing Steel Notes and Reinforcing Steel Bending Diagram see Index 521-480.

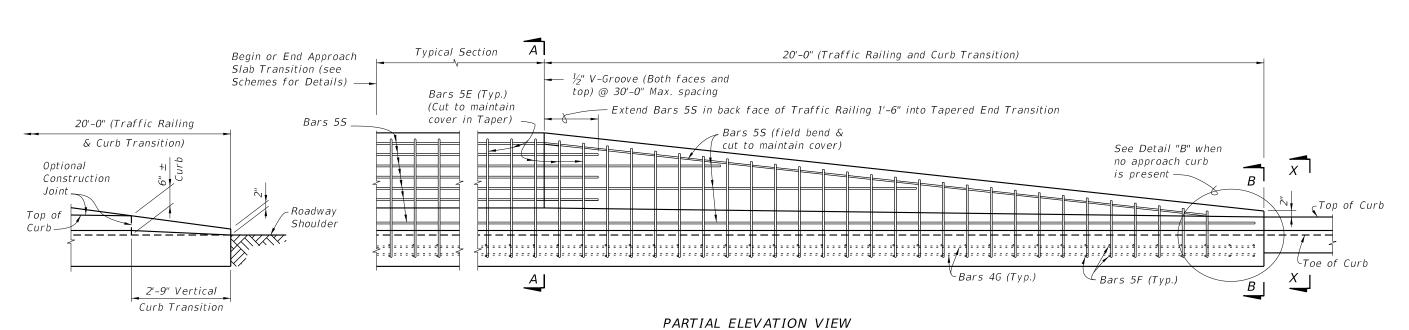
REVISION 07/01/09

DESCRIPTION:

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PARTIAL PLAN VIEW



DETAIL "B" TRANSITION TO NON-CURB APPROACH (Reinforcing Not Shown For Clarity)

TAPERED END TRANSITION = CROSS REFERENCES:

REVISION

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

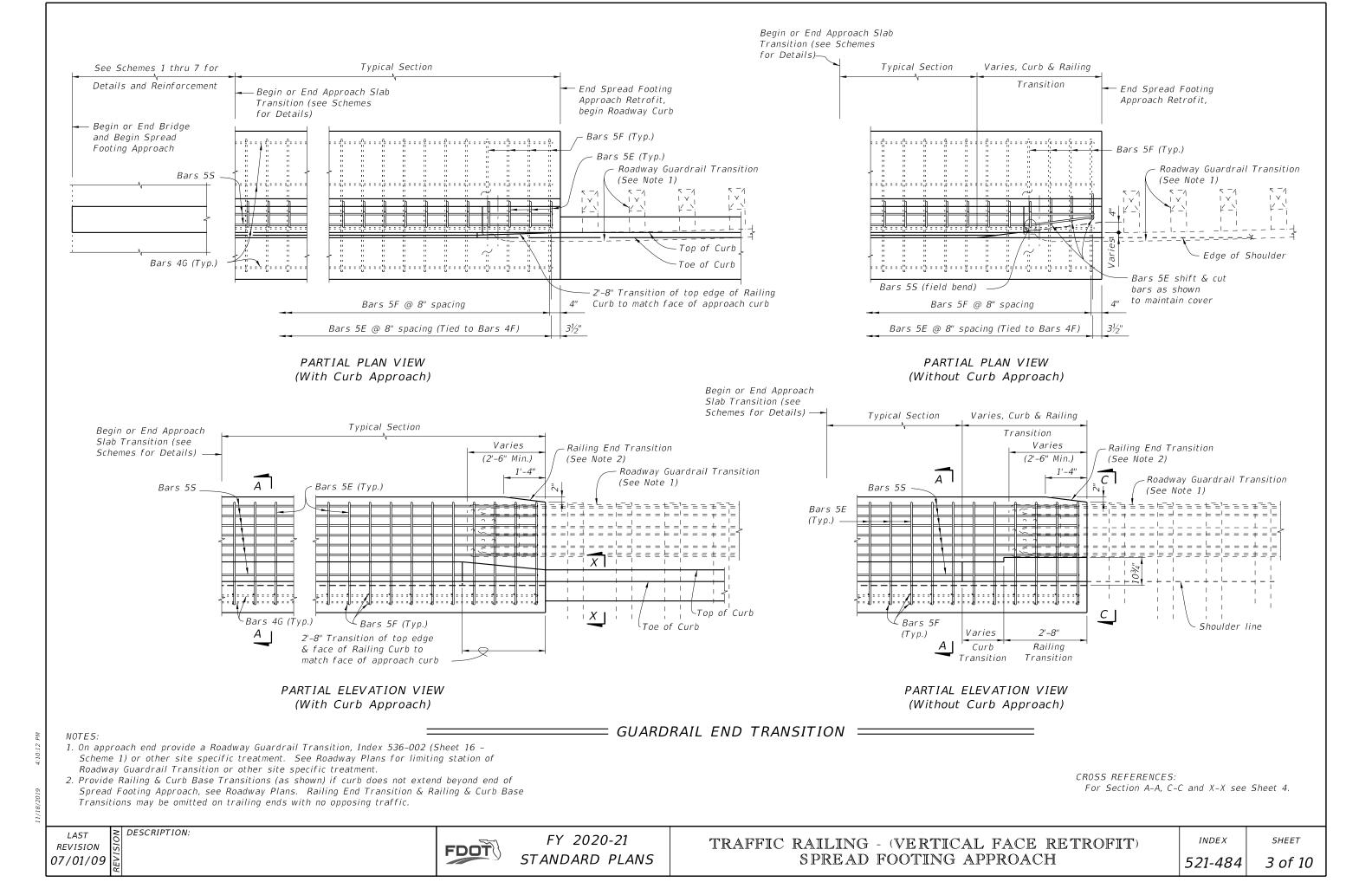
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

INDEX

For Section A-A, B-B and X-X see Sheet 4.

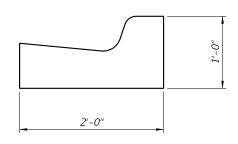
SHEET

07/01/09



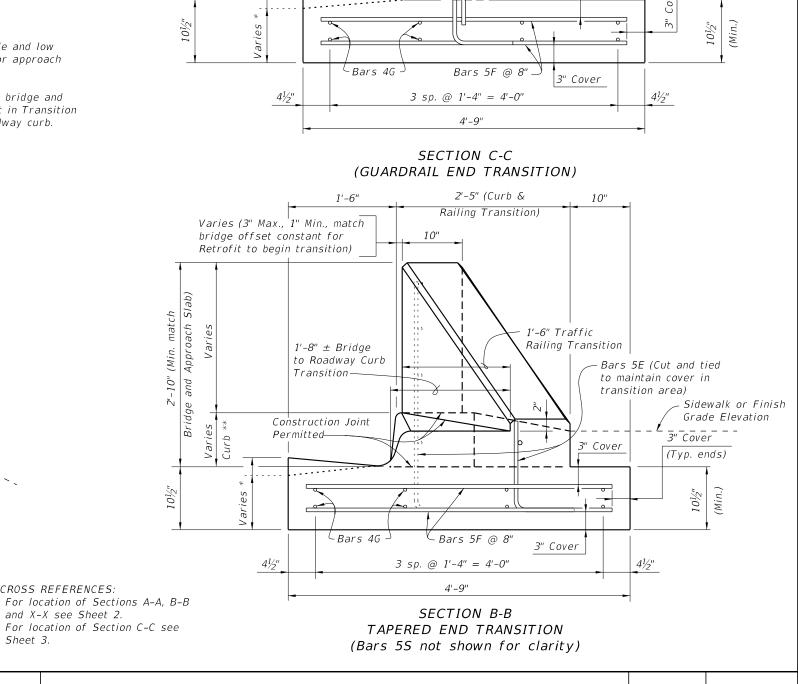
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES				
ITEM	UNIT	QUANTITY		
11 EM	OWIT	9" Curb		
Concrete - Typical Section	CY/Ft.	0.25		
Reinforcing Steel – Typical Section	Lb./Ft.	38		
Concrete - 20'-0" Tapered End Transition plus Footing	CY	4.57 Total		
Reinforcing Steel – 20'-0" Tapered End Transition plus Footing	Lb.	776 Total		

NOTE: Quantities are based on a 9" curb, no curb cross slope.



SECTION X-X (TYPICAL CURB, TYPE VARIES, TYPE F SHOWN) (See Index 520-001 and Plans for Details)

- * Match Cross Slope of high side and low side at begin or end bridge or approach
- ** Match curb height of adjacent bridge and approach slab. Adjust height in Transition area to match adjoining Roadway curb.



1'-1"

10"

1'-6"

Curb & Railing

Transition-

Slope *

Varies (3" Max., 1" Min., match

' (Min. match Approach Slab)

'-10" and

Bridge

bridge offset constant for Retrofit to begin Curb & Railing Transition) 2'-2"

Varies (0" to 2")

– End Bar 5E (field cut & shift to maintain cover)

Bars 5S (Typ.)

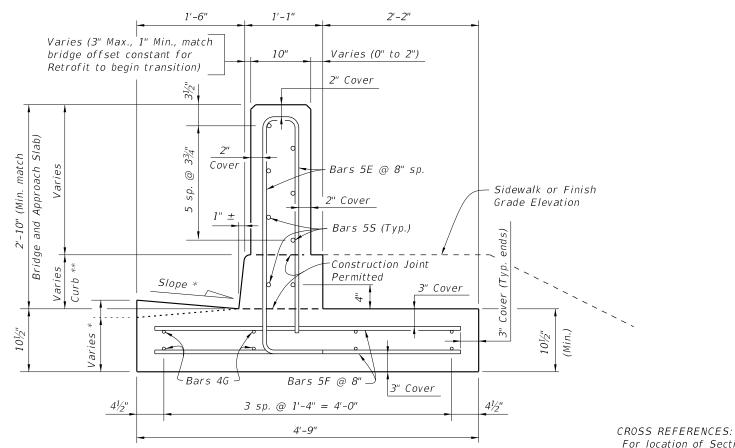
Permitted

Construction Joint

3" Cover

Sidewalk or Finish

Grade Elevation



SECTION A-A TYPICAL SECTION (9" Curb shown, 6" Curb similar)

FY 2020-21

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

INDEX 521-484

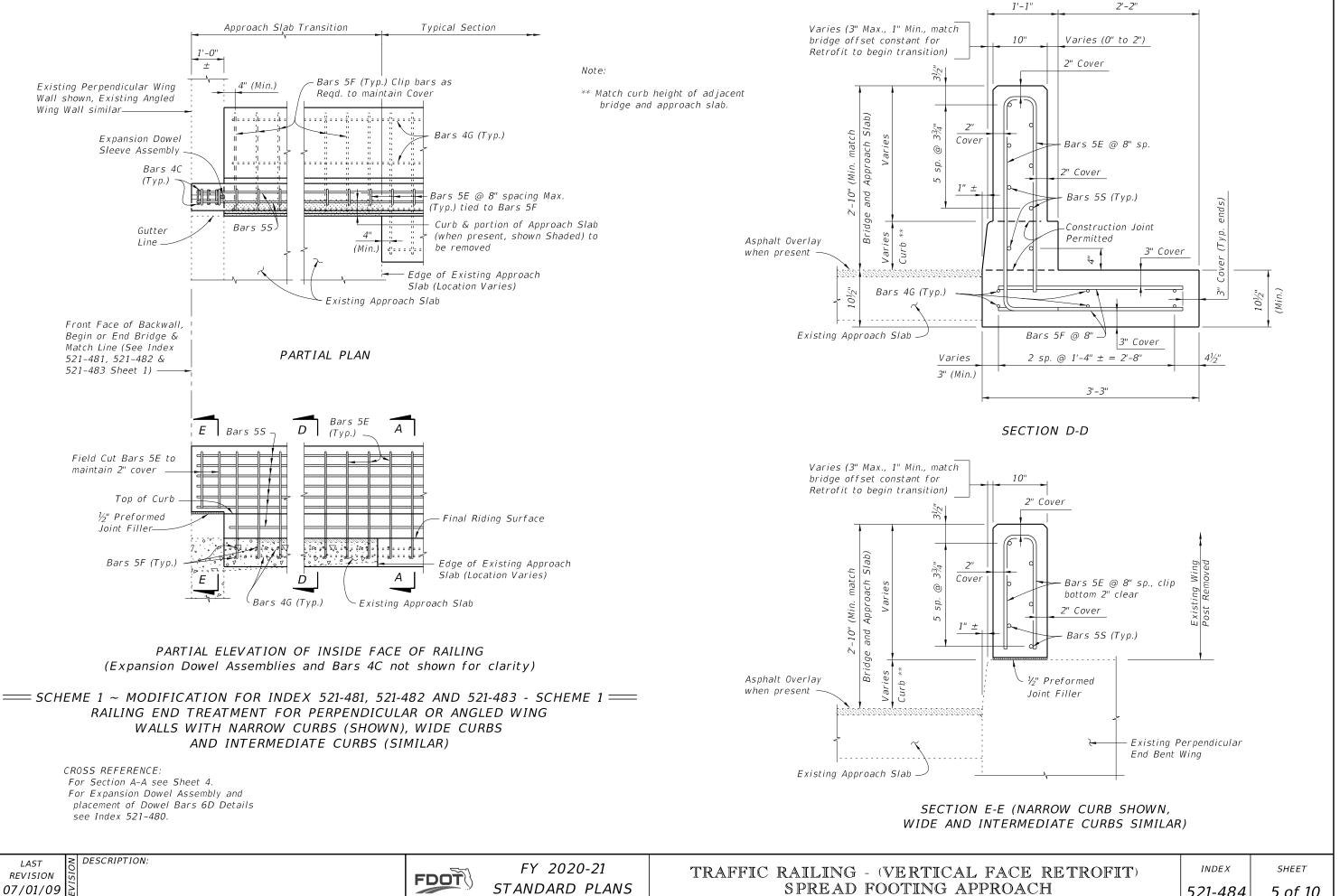
SHEET 4 of 10

REVISION 07/01/09

DESCRIPTION:

FDOT

Sheet 3.

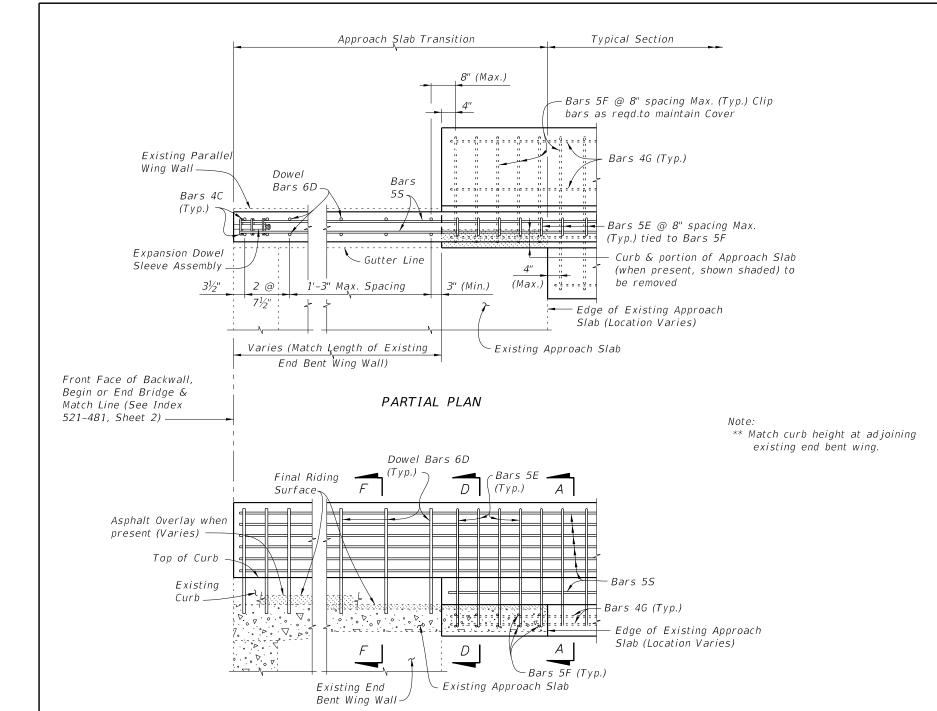


STANDARD PLANS

SPREAD FOOTING APPROACH

521-484

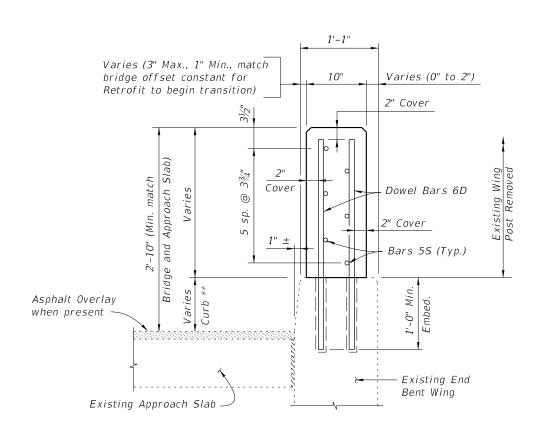
5 of 10



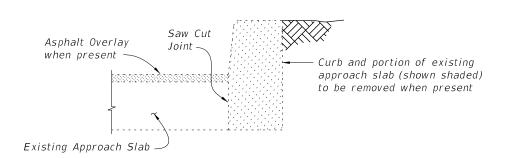
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

NOTES:

1. Remove existing concrete along saw cut joints. Existing reinforcing steel may be cut at joint or extended into new concrete. Exposed existing reinforcing not encased in new concrete shall be removed 1" below existing concrete surface and grouted over.



SECTION F-F



SECTION THRU EXISTING CURB AND APPROACH SLAB TO BE REMOVED (Free Standing Curb Similar)

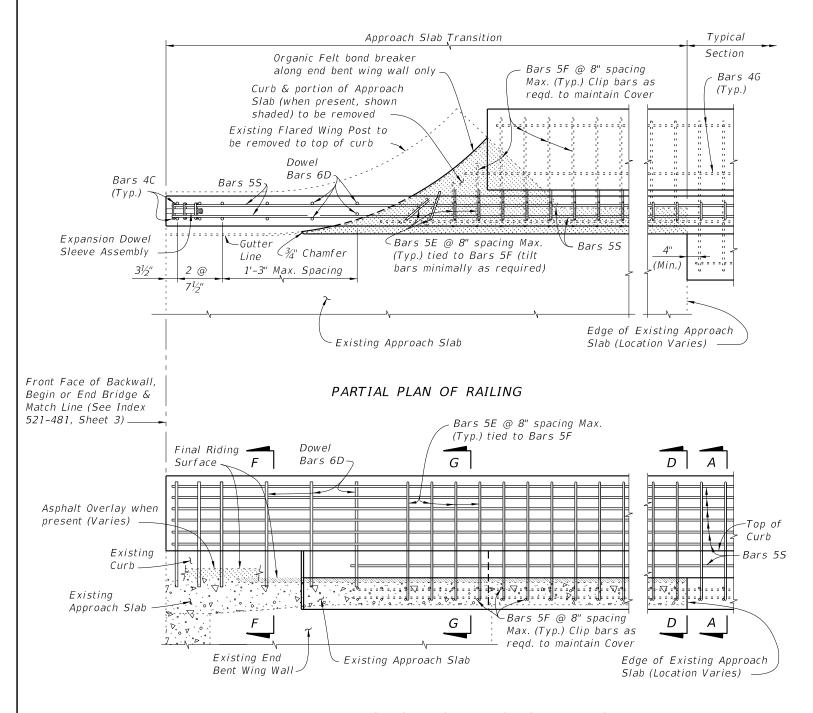
CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

LAST REVISION 07/01/09

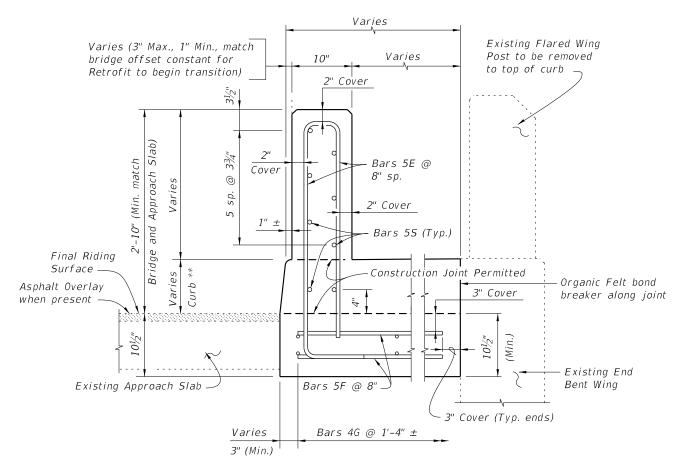
FDOT

FY 2020-21 STANDARD PLANS



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

SCHEME 3 ~ MODIFICATION FOR INDEX 521-481 SCHEME 3 ====== RAILING END TREATMENT FOR FLARED WING WALLS WITH NARROW CURBS



SECTION G-G

Note:

** Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4.

For Section D-D see Sheet 5. For Section F-F see Sheet 6.

For Expansion Dowel Assemblies Details and placement of Dowel Bars 6D see Index 521-480.

REVISION 11/01/16

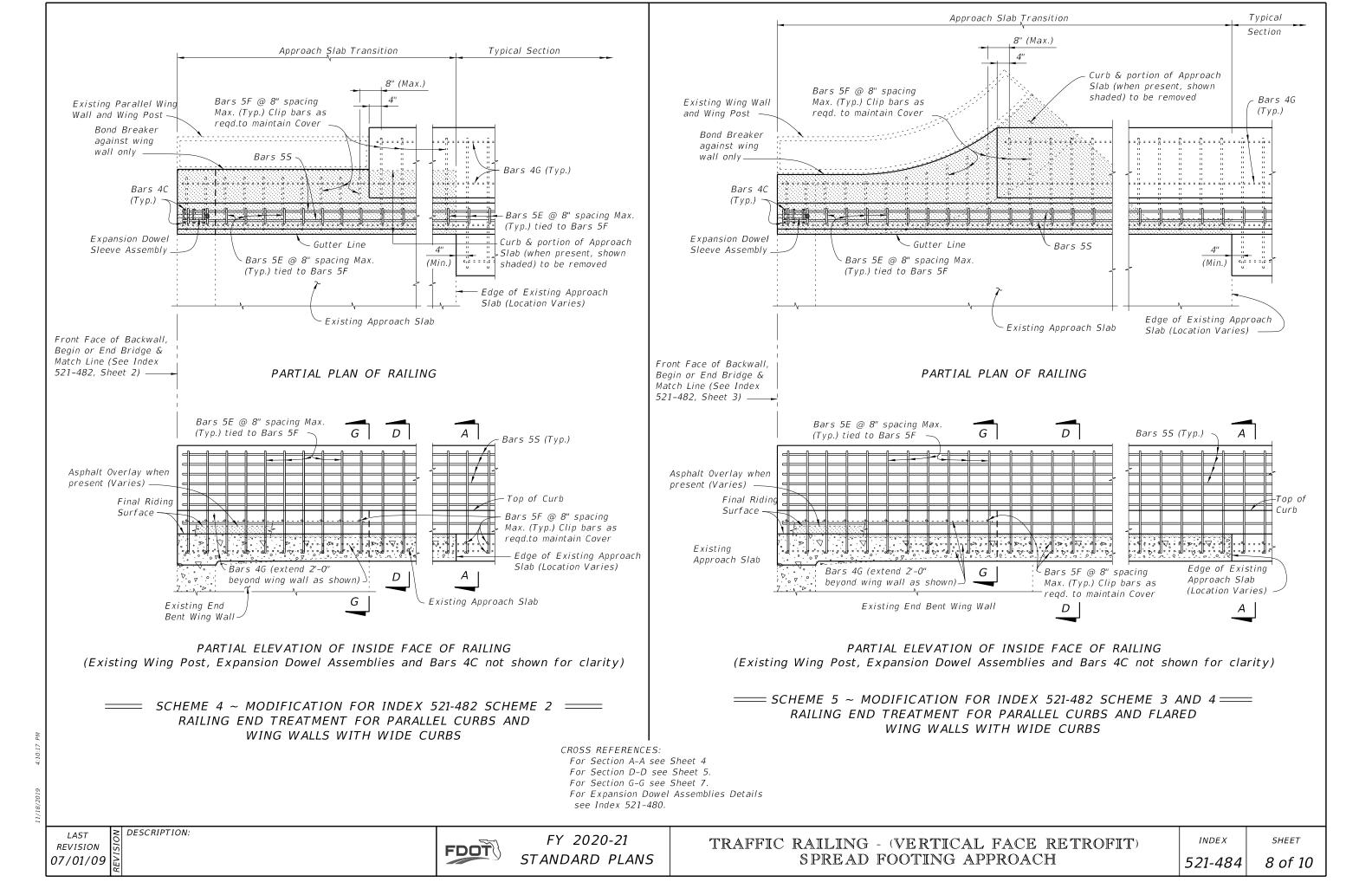
FDOT

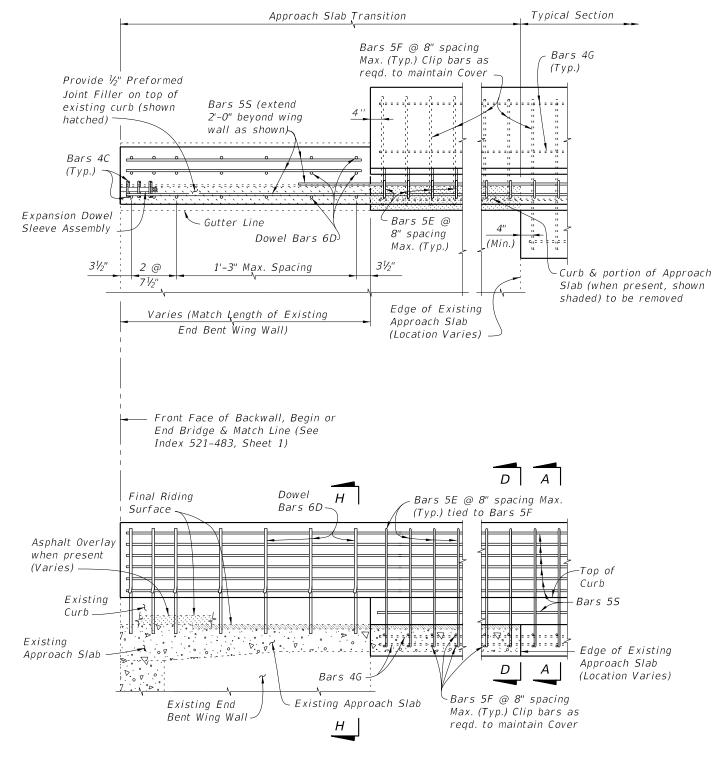
FY 2020-21 STANDARD PLANS

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

INDEX 521-484 SHEET

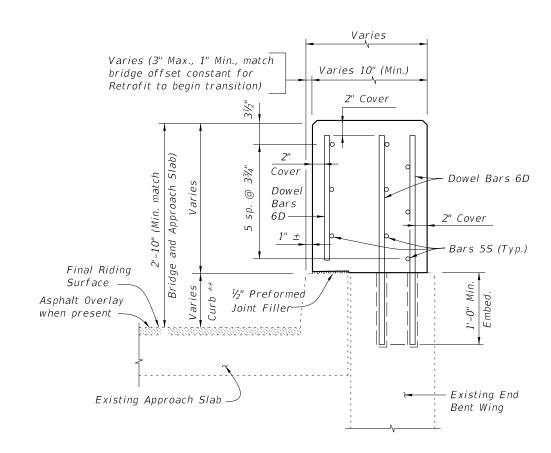
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PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 6 ~ MODIFICATION FOR INDEX 521-483 SCHEME 2 ===== RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH INTERMEDIATE CURBS



SECTION H-H

Note:

** Match curb height at adjoining existing end bent wing.

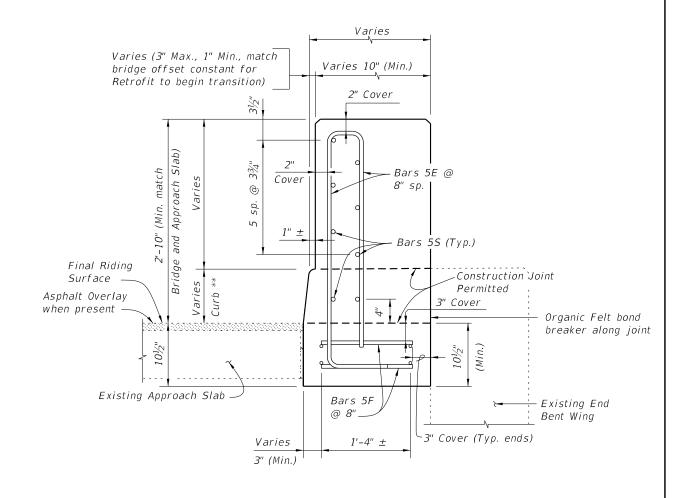
CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 521-480.

REVISION 07/01/09

DESCRIPTION:

FDOT



SECTION I-I

** Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section H-H see Sheet 9. For Expansion Dowel Assemblies and placement of Dowel Bars 6D Details see Index 521-480.

DESCRIPTION: REVISION 11/01/16

FDOT

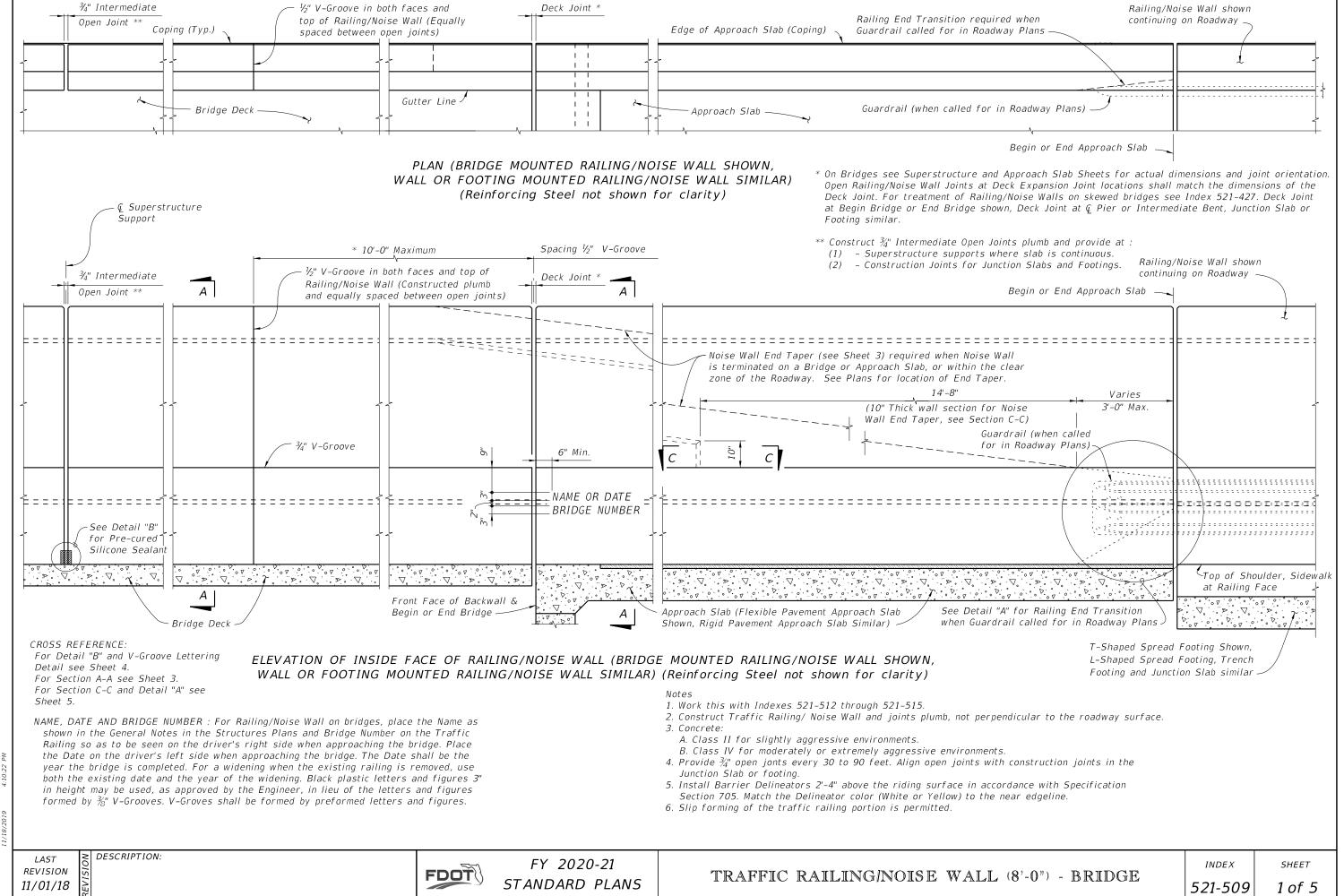
FY 2020-21 STANDARD PLANS

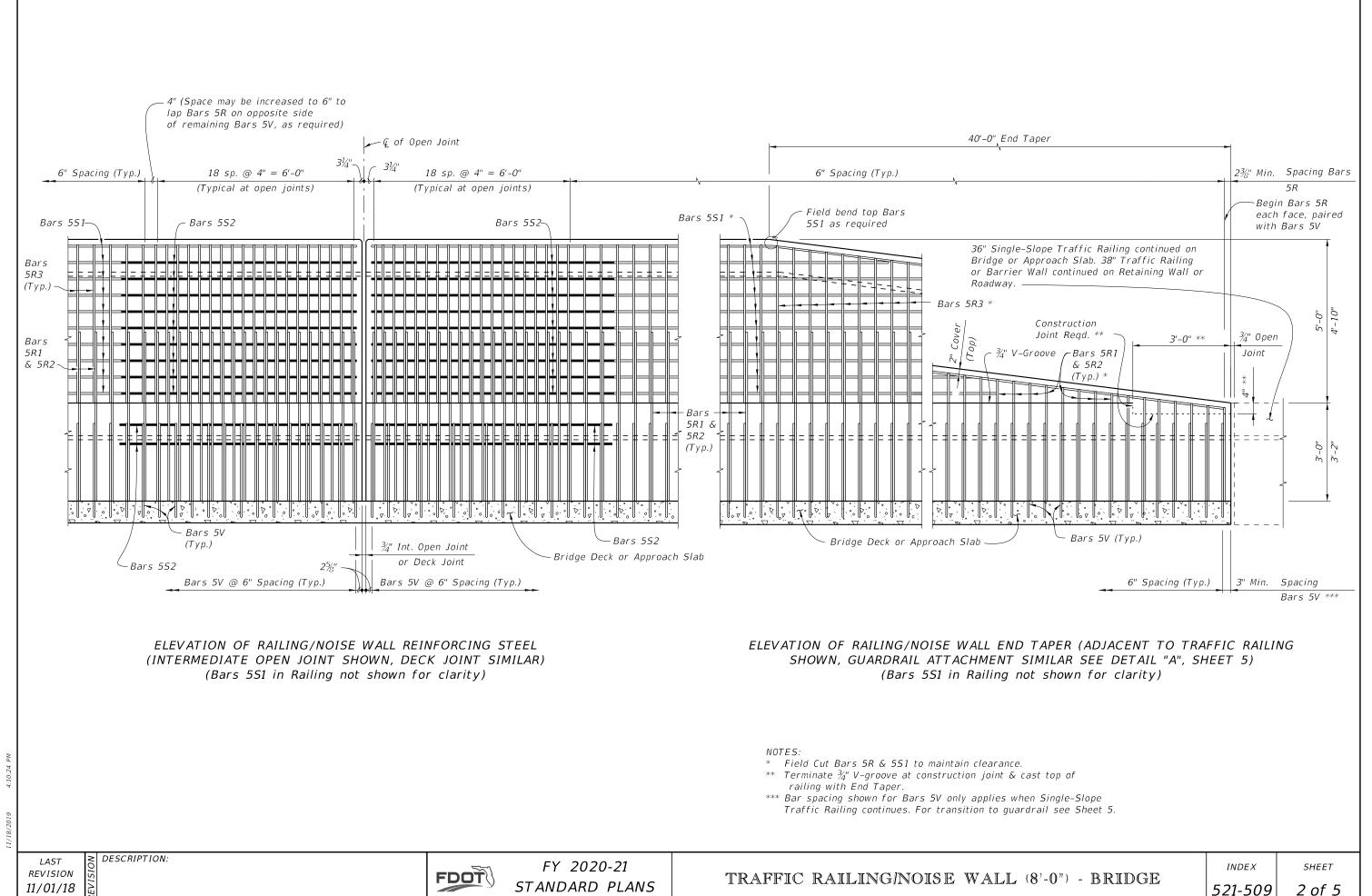
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

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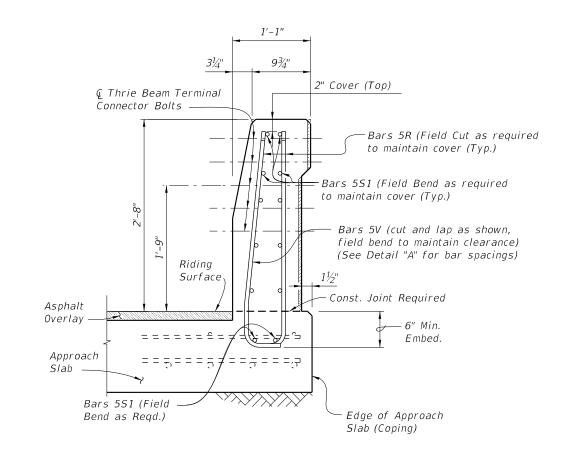


SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING/NOISE WALL (Section Thru Bridge Deck Shown, Section Thru Approach Slab Similar)

CROSS REFERENCE:

For locations of Section A-A see Sheet 1. For location of View B-B, see Sheet 5.

1. Bottom Bars 5S1 shown are part of the Traffic Railing/Noise Wall reinforcing. See Superstructure Sheets in the Plans for additional Bridge Deck Reinforcing.



VIEW B-B
END VIEW OF RAILING END TRANSITION FOR
GUARDRAIL ATTACHMENT AT END OF APPROACH SLAB
(Flexible Pavement Approach Slab Shown, Rigid Pavement Approach Slab Similar)

LAST REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

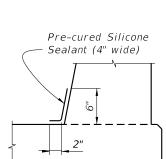
DESCRIPTION:

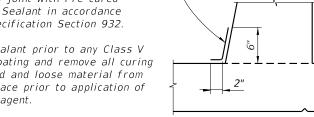
3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract

DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

ESTIMATED TRAFFIC RAILING/NOISE WALL QUANTITIES				
ITEM	UNIT	QUANTITY		
Concrete (Railing)	CY/LF	0.107		
Concrete (Noise Wall)	CY/LF	0.136		
Reinforcing Steel (Typical)	LB/LF	69.36		
Additional Reinf. @ Open Joint	LB	226.85		

(The above quantities are based on the bridge mounted typical section, 2% deck cross slope and railing on low side of deck.)





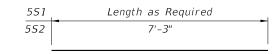
	Unit F	rice	for	the	Traffic	Raili	ing.	
_	AII	II DII	,		TION	A T	INTERNACRIATE	005

FD	To	{}
		У

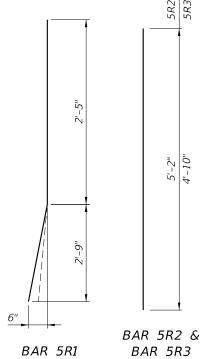
REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL					
MARK	SIZE	LENGTH			
R1	5	5'-2"			
R2	5	5'-2 ¹ / ₂ "			
R3	5	4'-10"			
51	5	As Reqd.			
52	5	7'-3"			
V	5	6'-6½"			

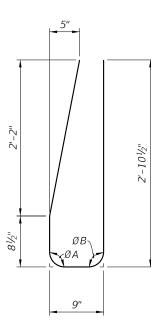
	BRIDGE CROSS-SLOPE		UTTER	HIGH C	GUTTER
CR			ØB	ØA	ØB
E :D	0% to 2%	90°	90°	90°	90°
BRIDGE	2% to 6%	93°	87°	87°	93°
B) M0	6% to 10%	96°	84°	84°	96°



BARS 5S1 & 5S2



(Field Cut and Bend for Railing End Transition)



STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut (Railing End Transition)

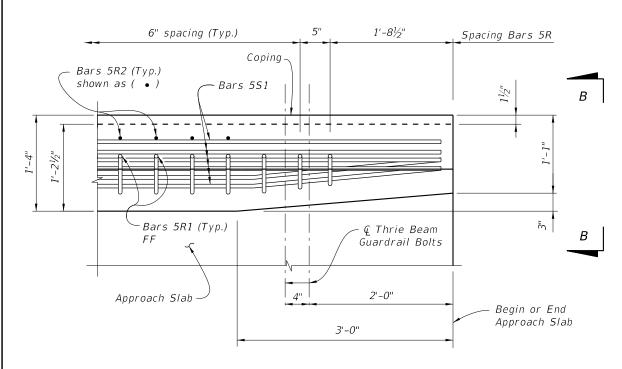
REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.
- 4. Bars 5S1 may be continuous or spliced at the construction joints. Lap splices for Bars 5R2 and 5S1 shall be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

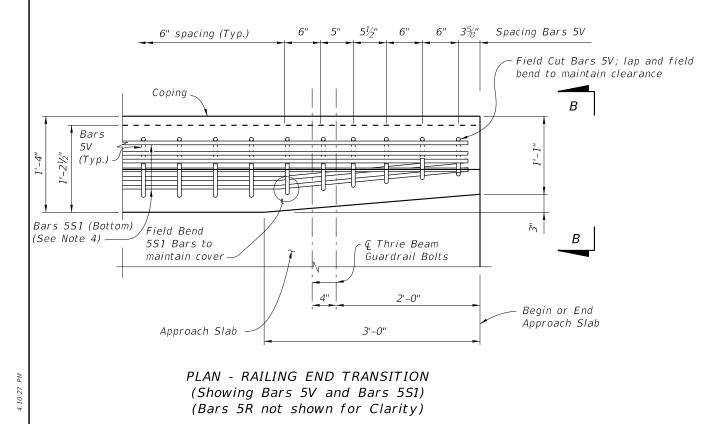
CROSS REFERENCE: For locations of Detail "B", see Sheet 1.

REVISION

11/01/18



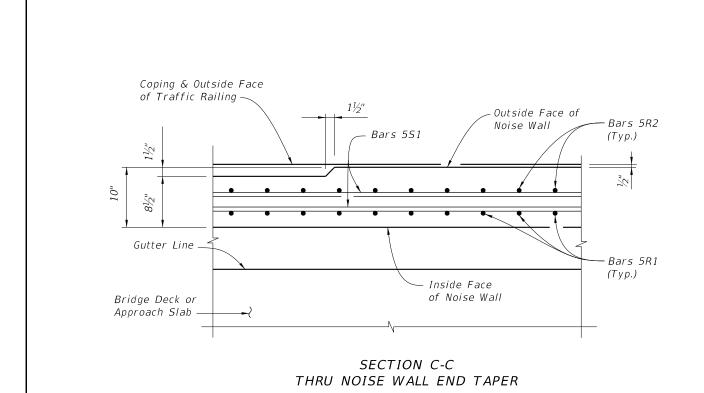
PLAN - RAILING END TRANSITION (Showing Bars 5R, and Bars 5S1) (Bars 5V & Noise Wall Reinforcement not shown for Clarity)



DETAIL "A"

DETAIL "A" NOTES:

- 1. Begin placing Railing Bars 5V at the railing end and proceed toward the guardrail (thrie beam) terminal connector to ensure placement of guardrail bolt holes. Pair Bars 5R with Bars 5V as shown. Clearance of Bars 5R & 5V to guardrail bolt holes shall be checked to prevent cutting of bars if holes are to be drilled. Shift bars locally where conflicts occur.
- For Guardrail connection details see Index 536-001.
- Omit Railing End Transition if a 36" Single-Slope Traffic Railing is used beyond the End Taper. See the
- 4. Field cut Bars 5R2 to maintain cover. Field cut Bars 5V and lap as necessary to maintain cover; field cut & bend Bars 5R1 front leg (more plumb) to maintain cover and tie to S1 Bars.



CROSS REFERENCE: For location of Detail "A" see Sheet 1. For location of Section C-C see Sheet 1.

For View B-B see Sheet 3.

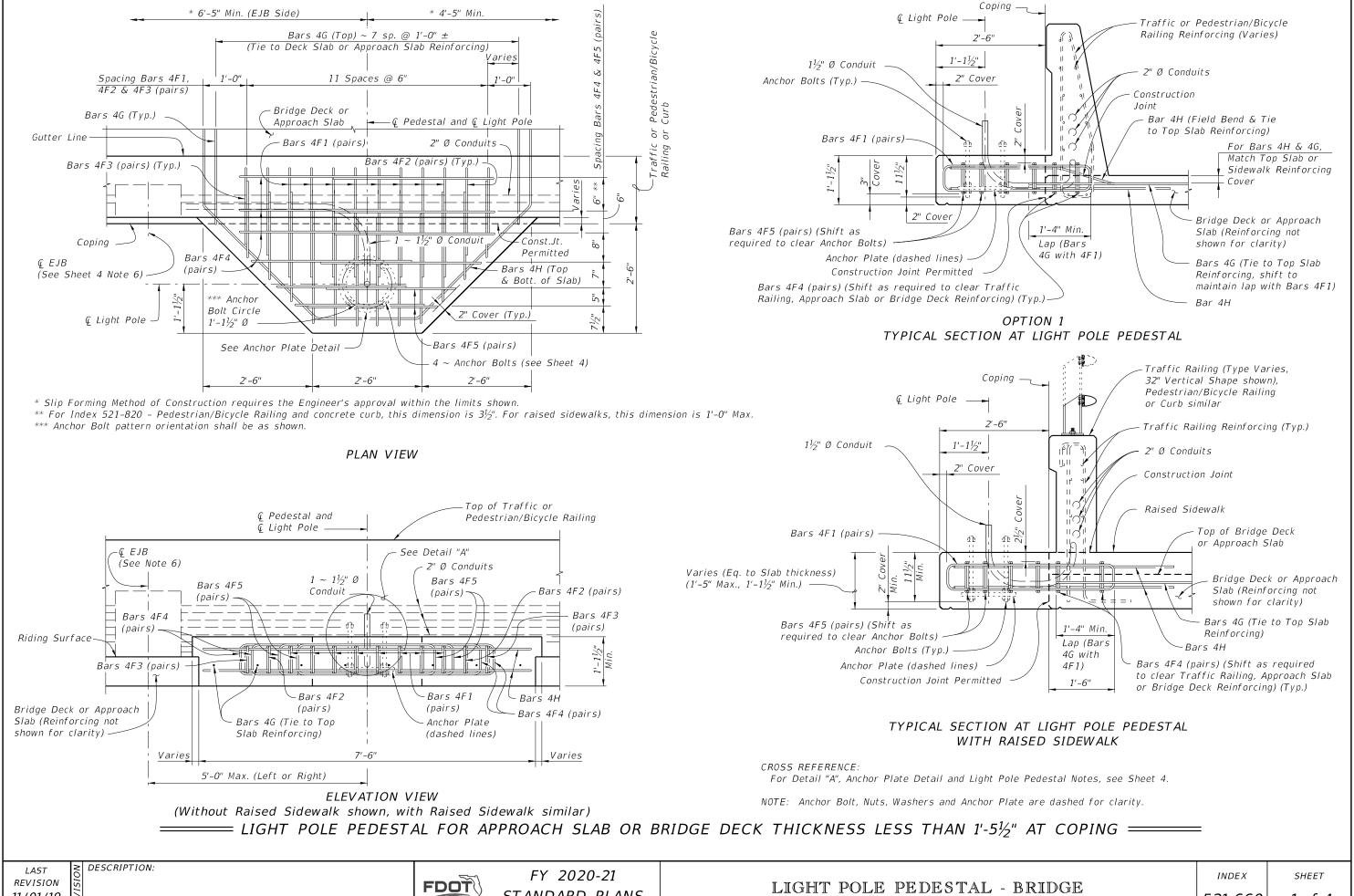
DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS INDEX

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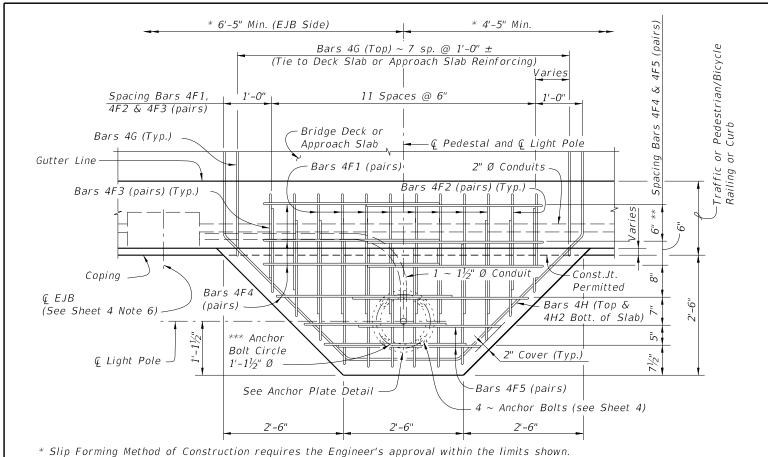
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STANDARD PLANS

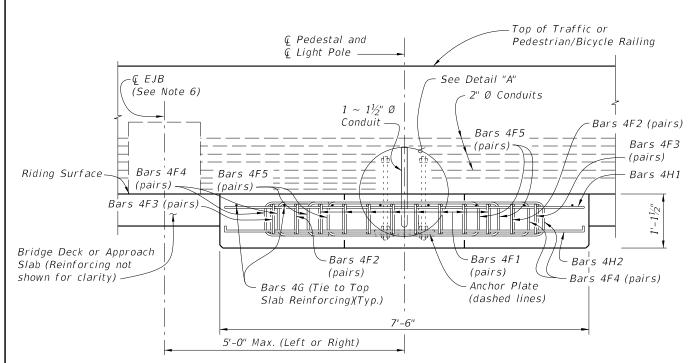
521-660 1 of 4



** For Index 521-820 - Pedestrian/Bicycle Railing and concrete curb, this dimension is 3½". For raised sidewalks, this dimension is 1'-0" Max.

*** Anchor Bolt pattern orientation shall be as shown.

PLAN VIEW



OPTION 2 - ELEVATION VIEW

1'-11/2" 2" Ø Conduits € Light Pole Construction Joint $1\frac{1}{2}$ " Ø Conduit Bars 4F1 & 4F2 Bars 4H2 Tie Bars 4F5 Bars 4H1 & 4G to bottom slab (pairs) reinforcement Bars 4F4 (pairs) (Typ.) 1'-2" Maintain pedestal depth Construction to edge of Girder Flange (Min.) Joint Permitted 1'-4" (Min.) Lap Bars 4G with Bars 4F1 OPTION 2 - TYPICAL SECTION AT LIGHT POLE PEDESTAL

← Coping

2'-6"

(Approach Slab Similar)

CROSS REFERENCE:

For Detail "A", Anchor Plate Detail and Light Pole Pedestal Notes, see Sheet 4.

NOTE: Anchor Bolt, Nuts, Washers and Anchor Plate are dashed for clarity.

= LIGHT POLE PEDESTAL FOR APPROACH SLAB OR BRIDGE DECK LESS THAN 1'-5 $^1\!\!/_2$ " AT COPING OPTION 2 ========

REVISION 11/01/18

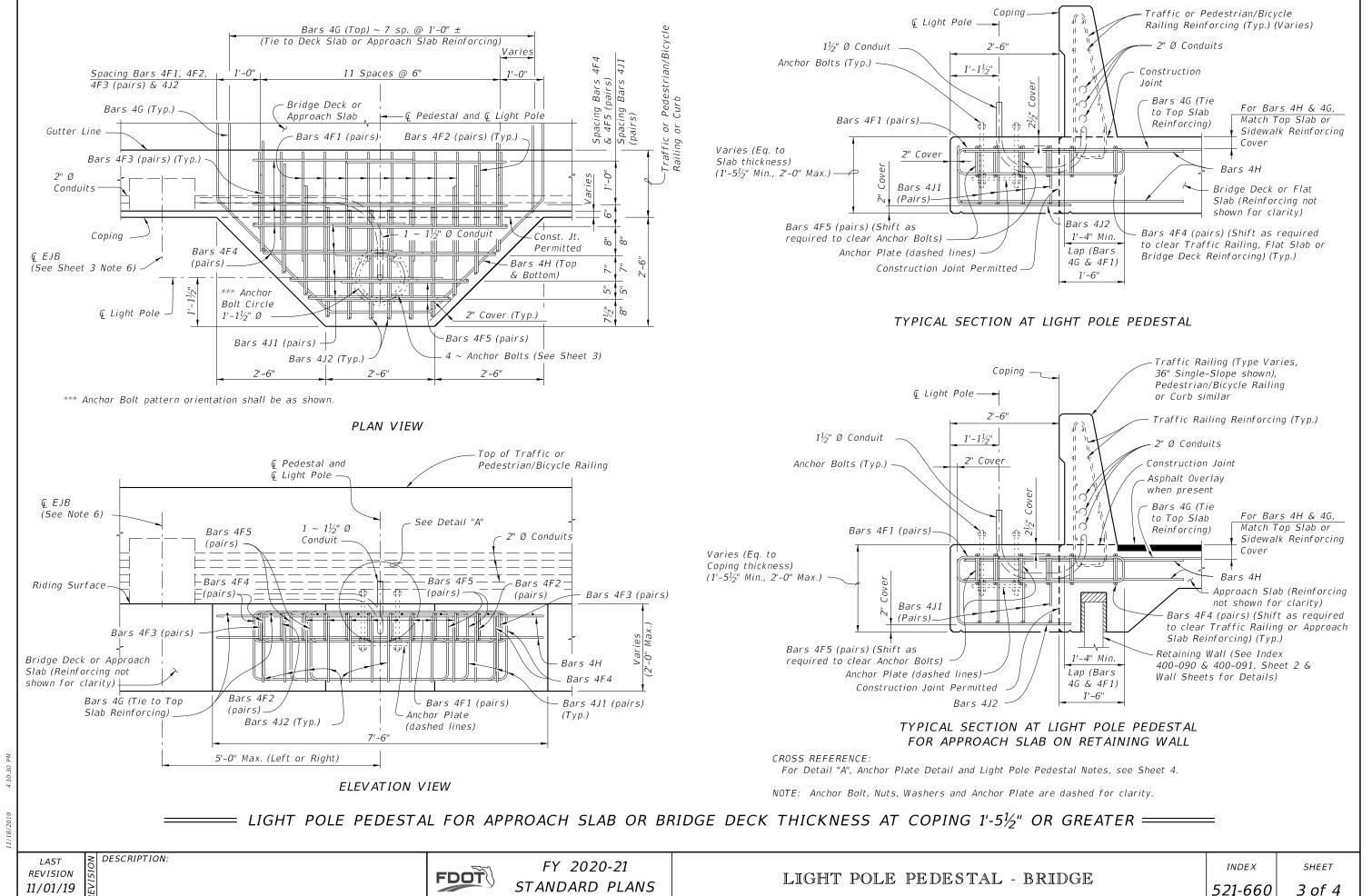
DESCRIPTION:

FY 2020-21 STANDARD PLANS

LIGHT POLE PEDESTAL - BRIDGE

INDEX *521-660*

SHEET 2 of 4



CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

REINFORCING STEEL NOTES:

 $1'-1\frac{1}{5}$ " Ø bolt hole circle

 $4 \sim (Bolt Dia. + \frac{1}{16})$ Ø

Holes equally spaced

Light Pole Base

Plate (Level)

Leveling Nut

Anchor Bolts (See

Notes 4 & 5)

- a. When Pedestal is attached to Pedestrian/Bicycle Railing Index 521-820 or an 8" wide concrete curb and the Bridge Deck or Approach Slab thickness is less than 1'-1 $\frac{1}{2}$ ", Bars 4F3 shall have leg length and bar length shown in parentheses.
- b. The number of bars shown in parentheses is for Bars 4F4 when Pedestal is attached to Pedestrian/Bicycle Railing - Index 521-820 or an 8" wide concrete curb, and the Bridge Deck or Approach Slab thickness is less than 1'-11//".

1'-31/5'

111/5"

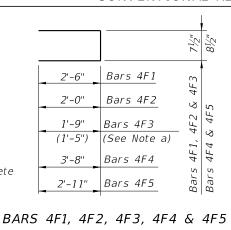
ANCHOR PLATE DETAIL

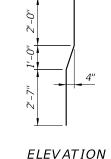
Washer

(Typ.)

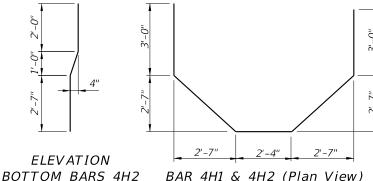
- c. Lap Splices for Bars 4F1, 4F2 & 4F3 shall be a minimum of 1'-4". Lap Splices for Bars 4F4 & 4F5 shall be minimum of 1'-8".
- d. Bars 4J1 and 4J2 are not required when Pedestal thickness is less than $1'-5\frac{1}{2}''$. Field trim height of bars to maintain cover when Pedestal thickness is less than 2'-0". Field trim length of Bars 4J2 on Retaining Wall Coping to maintain cover.
- e. All bar dimensions in the bending diagrams are out to out.

Light Pole-

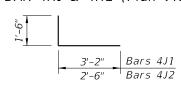




(For Option 2)



BAR 4H1 & 4H2 (Plan View)



BARS 4J1 & 4J2

()) See	Reinforcing	Steel	Note	а	&	b.
-----	-------	-------------	-------	------	---	---	----

SIZE

4

4

4

4

4

MARK F 1

F2

F3

F4

F5

G

Н

J 1

J2

BILL OF REINFORCING STEEL

LENGTH

5'-8"

4'-8"

4'-2"

(3'-6'')

8'-3"

6'-7"

6'-0"

15'-8"

4'-8"

4'-0"

NOTES

С

С

а, с

b, c

С

d

d

NO. REQD.

16

4

4

8

(6)

4

8

2

8

12

6'-0"

BAR 4G

LIGHT POLE PEDESTAL NOTES

- 1. Concrete and Reinforcing Steel required for the construction of the Pedestal shall meet the same requirements as the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.
- 2. Light Pole Pedestal may be used with the following: Index 521–422 – Traffic Railing (42" Vertical Shape), Index 521-423 - Traffic Railing (32" Vertical Shape), Index 521-427 - Traffic Railing (36" Single-Slope), Index 521-428 - Traffic Railing (42" Single-Slope), Index 521-820 - Pedestrian/Bicycle Railing, Index 515-021 - Pedestrian/Bicycle Bullet Railing for Traffic Railing or
 - Index 515-509 Traffic Railing /Noise Wall Bridge.
- 3. Unless otherwise noted, Traffic Railing (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other Traffic Railings or Pedestrian/Bicycle Railing are similar.

4. ANCHOR BOLTS:

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole configurations shown on Index 715-002.

Anchor Bolt Diameter: See Table 1 Anchor Bolts: ASTM F1554 Grade 55. Nuts: ASTM A563 Grade A. Heavy-Hex.

Washers: ASTM F436 Type 1.

Anchor Plate: ASTM A709 (Grade 36) or ASTM A36.

Coating: Galvanize all Nuts, Bolts Washers, in accordance with ASTM F2329. Galvanize plates in accordance with ASTM A123.

The Contractor is responsible for ensuring the anchor bolt configuration is compatible with the light pole base plate. Submit modifications of the anchor bolt design to the Engineer for approval.

- 5. Install Anchor Bolts plumb.
- 6. For Conduit, Embedded Junction Boxes (EJB), Expansion/Deflection Fitting and adjacent Reinforcing Steel Details, see Utility Conduit Detail Sheets.
- 7. PAYMENT: The cost of Wire Screen, Anchor Bolts, Nuts, Washers and Anchor Plates shall be included in the Bid Price for Light Poles. The cost of all Labor, Concrete and Reinforcing Steel required for the Construction of the Pedestals, and Miscellaneous Hardware required for the completion of the Electrical System, shall be included in the Bid Price for the Traffic Railing or Pedestrian/Bicycle Railing the Pedestal is attached to.

2	(9½" Minimim)	7			
	L				

Wire Screen (See Spec. 649-6)

Bottom of Anchor Plate

BRIDGE DECK HEIGHT (Ft.)* WIND ARM SPEED LENGTH DESIGN MOUNTING HEIGHT (MPH) (Ft.) 40 Ft. 45 Ft. 50 Ft. 120 75 ≤ 15 75 75 140 ≤ 15 75 75 75 45** 160 8 & 10 75 75 160 12 & 15 25**

TABLE 1 - DESIGN LIMITATIONS FOR ANCHOR BOLTS (1" Dia.)

- * Above natural ground or MLW.
- ** Use $1\frac{1}{4}$ " diameter Anchor Bolt for Bridge Deck Height greater than shown, in Table 1, up to 75'.

	ESTIMATED LIGHT POLE PEDESTAL QUANTITIES PER LIGHT POLE PEDESTAL				
ITEM UNIT QUANTITY					
	Concrete Per Pedestal Thickness	1 (V/In			
	Reinforcing Steel	LB	195 (182)		

(The Reinforcing Steel quantity shown in parenthesis is for a Pedestal attached to Pedestrian/Bicycle Railing - Index 521-820 with Bridge Deck or Approach Slab thinner than 1'-11/2". Add 59 Lbs. for Bars 4J1 & 4J2 when Pedestal Thickness is 1'-5½" or greater)

DETAIL "A"

CROSS REFERENCE: For location of Detail "A" see Sheets 1,2 and 3.

DESCRIPTION:

Anchor Plate

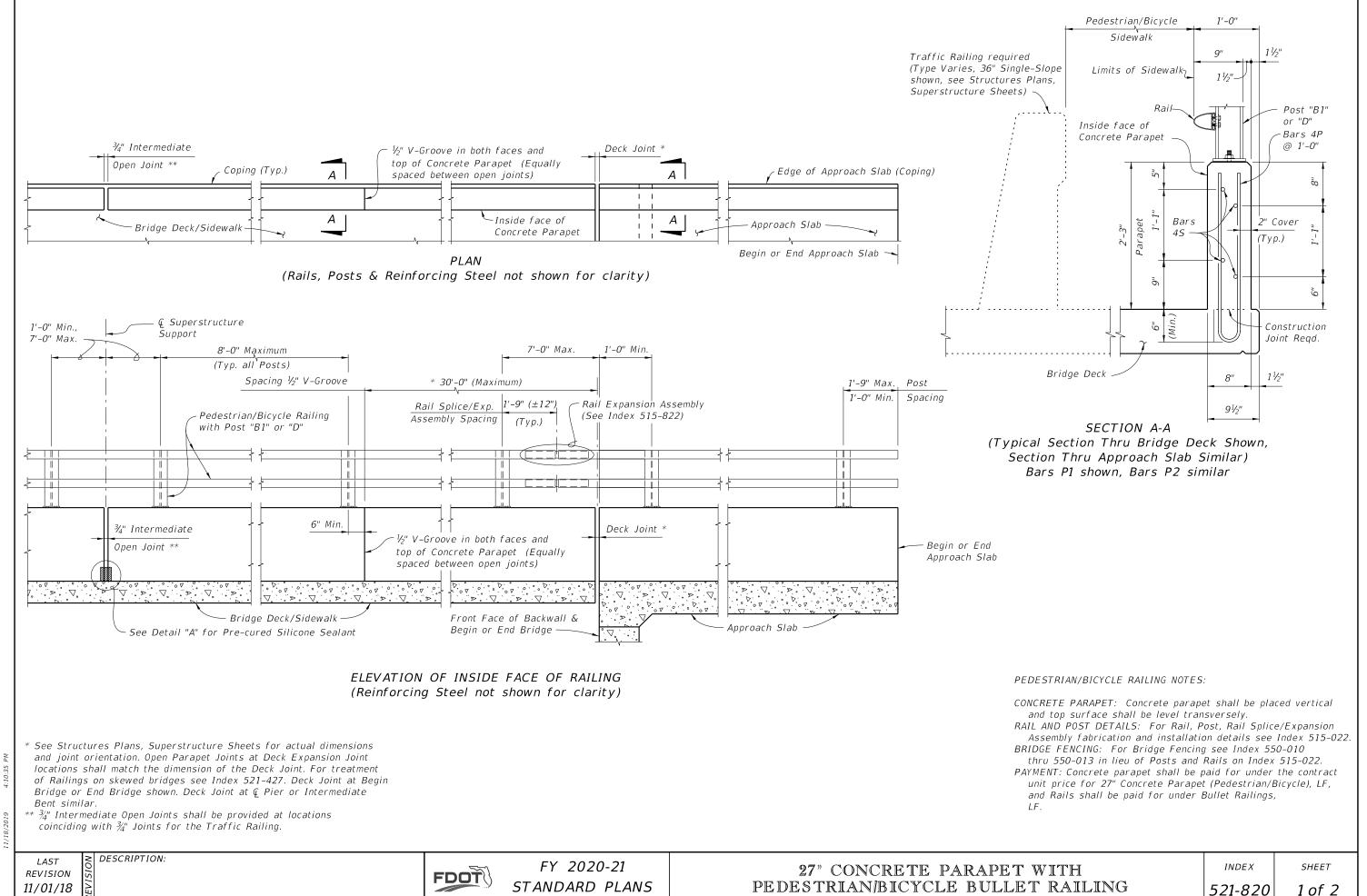


FY 2020-21 STANDARD PLANS

LIGHT POLE PEDESTAL - BRIDGE

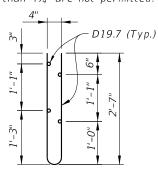
INDEX

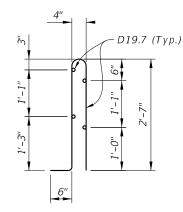
SHEET



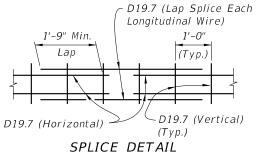
ALTERNATE REINFORCING (WELDED WIRE REINF.) DETAILS

NOTE: Place wire panels to minimize the end overhang. End Overhangs greater than 4¾" are not permitted.





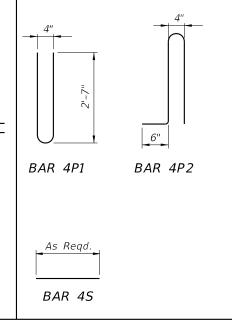
__ WELDED WIRE REINFORCEMENT (WWR) ___



(Between WWR Sections)

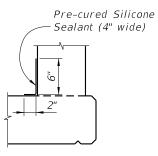
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL

MARK	SIZE	LENGTH
P1	4	5'-6"
P2	4	6'-0"
S	4	As Read



REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the parapet on a retaining wall shall be the same as detailed above for a 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Bars 4P2 may be used in lieu of Bars 4P1.
- 6. At the option of the Contractor deformed WWR may be used in lieu of all Bars 4P or 4P2 and 4S.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTE:

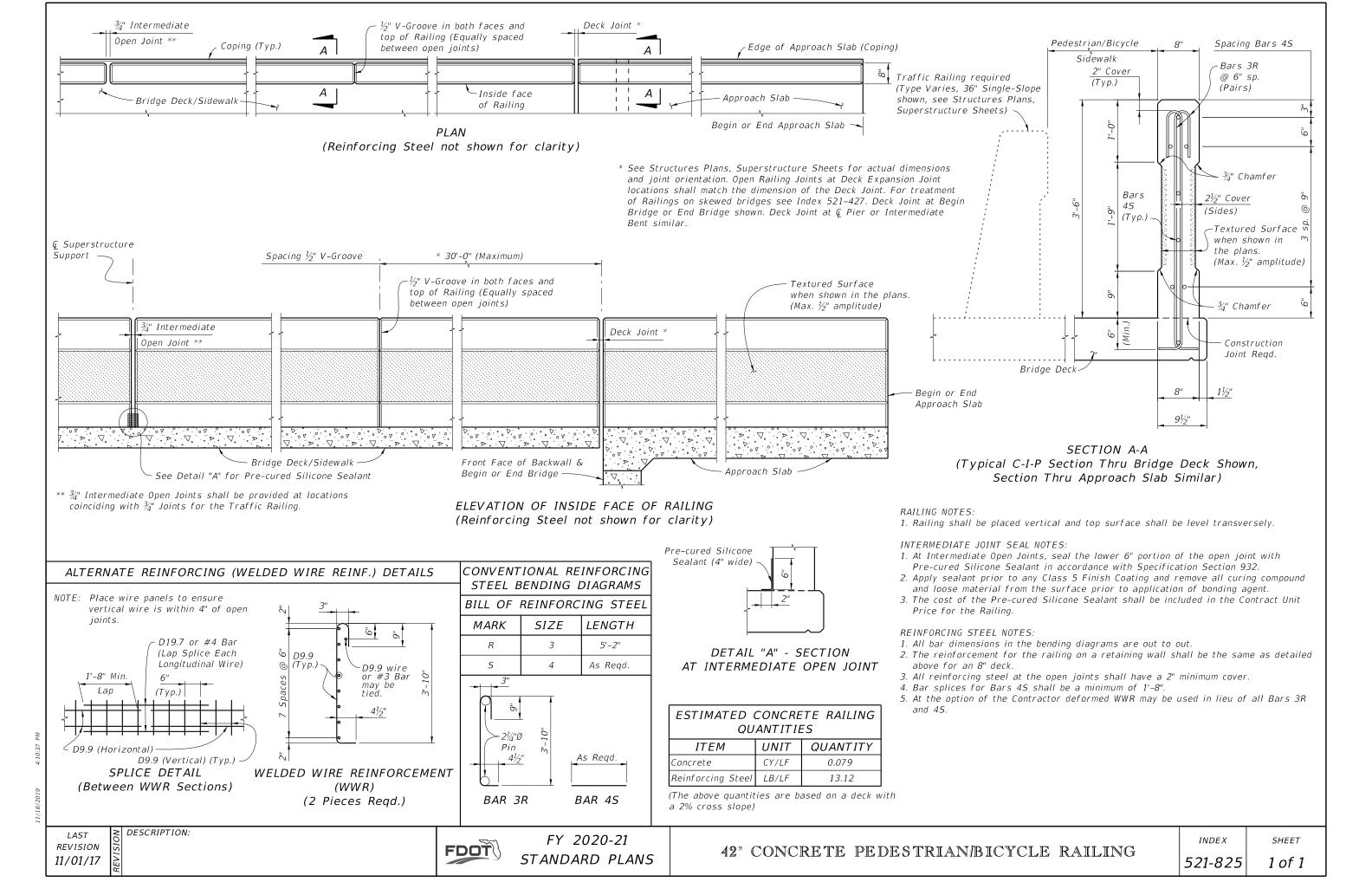
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant meeting the requirements of Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Concrete Parapet.

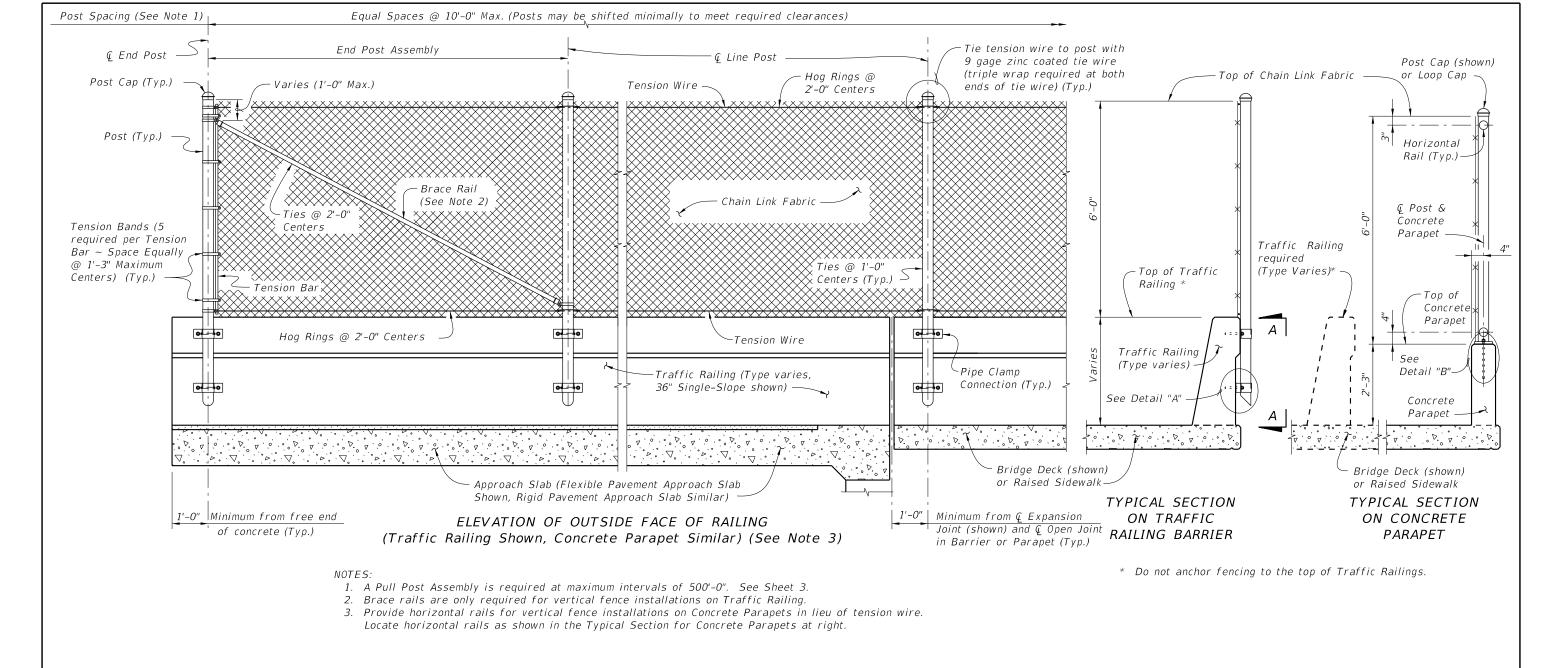
ESTIMATED CONCRETE PARAPET QUANTITIES				
ITEM	UNIT	QUANTITY		
Concrete	CY/LF	0.056		
Reinforcing Steel (P1 & S)	LB/FT	6.35		
Reinforcing Steel (P2 & S)	LB/FT	6.68		

(The above quantities are based on a deck with a 2% cross slope)

DESCRIPTION:







FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

CONCRETE PARAPET DETAILS:

DESCRIPTION:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace rails and bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, post and loop caps, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, bearing pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

For Pull Post Assembly Detail for Traffic Railings see Sheet 3.

For Pull Post Assembly Detail for Concrete Parapets and Detail "B" see Sheet 4.

LAST REVISION 11/01/17

FDOT

FY 2020-21 STANDARD PLANS

BRIDGE FENCING (VERTICAL)

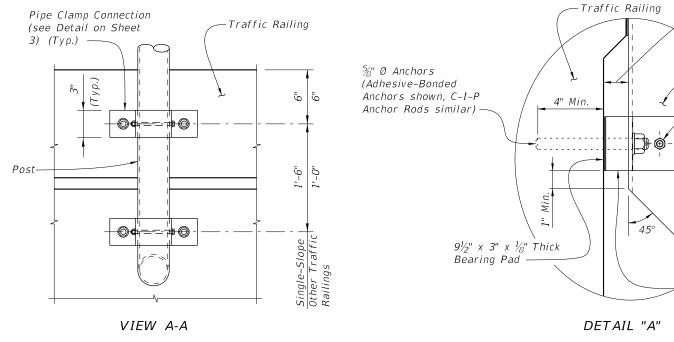
INDEX 550-010

SHEET

1 of 4

	TABLE OF CHAIN LINK FENCE COMPONENTS					
COMPONENT ASTM DESIGNATION			COMPONENT INFORMATION			
	Posts	F1083	Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade			
	Chain Link Fabric (2" mesh with twisted	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating			
ets	top and knuckled bottom selvage)	A491	Aluminum Coated Steel - 9 gage (coated wire diameter)			
Traffic Railings and Concrete Parapets		F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b			
ic Raı rete ı	Tie Wires	F626	Zinc Coated Steel Wire - 9 gage			
Traff Conc	Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)			
and	Tension Bars	F626	$rac{3}{16}$ " (Min. thickness) x $rac{3}{4}$ " (Min. width) x 5'-10" (Min. height) Steel Bars			
	Tension Bands	F626	14 Gage (Min. thickness) x ¾" (Min. width) Steel Bands			
	Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)			
	Horizontal Rails	F 1083	Galvanized Steel Pipe – $2\frac{1}{2}$ " NPS, Schedule 40 Regular Grade			
e S	Expansion Rails	F1083	Galvanized Steel Pipe - 2" NPS, Schedule 40 Regular Grade			
Concrete Parapets	Bolts	A307	$^{1}\!\!\!/_{4}$ " Ø x $^{4}\!\!\!/_{4}$ " Hex Head Bolts for Expansion Rail Connections			
CC Pè	Nuts	A563	Hex Nuts for Expansion Rail Connections			
	Washers	F 436	Flat Washers for Expansion Rail Connections			
gs	Tanaian Mina	1024 6 1017	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating			
ailin	Tension Wire	A824 & A817	Type I (Aluminum Coated Steel Wire) - 7 gage			
Traffic Railings	Hog Rings	F626	Zinc Coated Steel Wire - 12 gage			
Traf	Brace Rails	F1083	Galvanized Steel Pipe - $1\frac{1}{4}$ " NPS, Schedule 40 Regular Grade			

TABLE OF POST ATTACHMENT COMPONENTS				
COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe Clamps		A36 or A709 Grade 36	1/4" Steel PL	
Base Plates		A36 or A709 Grade 36	¾" Steel P_	
Shim Plates		A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be ¾" Ø	
Spacers		-	Plate thickness varies based on traffic railing type (See Detail "A")	
Slamp	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim 5\!\!\!/\!\!/$ 0 x 6" (no spacer) or $5\!\!\!/\!\!/$ 0 x (6" + spacer thickness)	
Pipe Clamp Connection	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x 6" (no spacer) or $\frac{7}{8}$ " Ø x (6" + spacer thickness)	
Base Plate Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods \sim $7_8^{\prime\prime}$ Ø x $147_2^{\prime\prime}$	
	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x 14 $\frac{1}{2}$ "	
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections	
Washers		F436	Flat Washers for Pipe Clamp and Base Plate Connections	
Bearing Pads (Plain Neoprene)		-	In accordance with Specification Section 932 for Ancillary Structures	



POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

WELDING:

R Spacer thickness

¾" Ø Bolt

- Pipe Clamp

R Spacer must be manufactured

from an incompressible material

(i.e. steel or aluminum)

Post

 $(1\frac{1}{2}$ " for Single-Slope)

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

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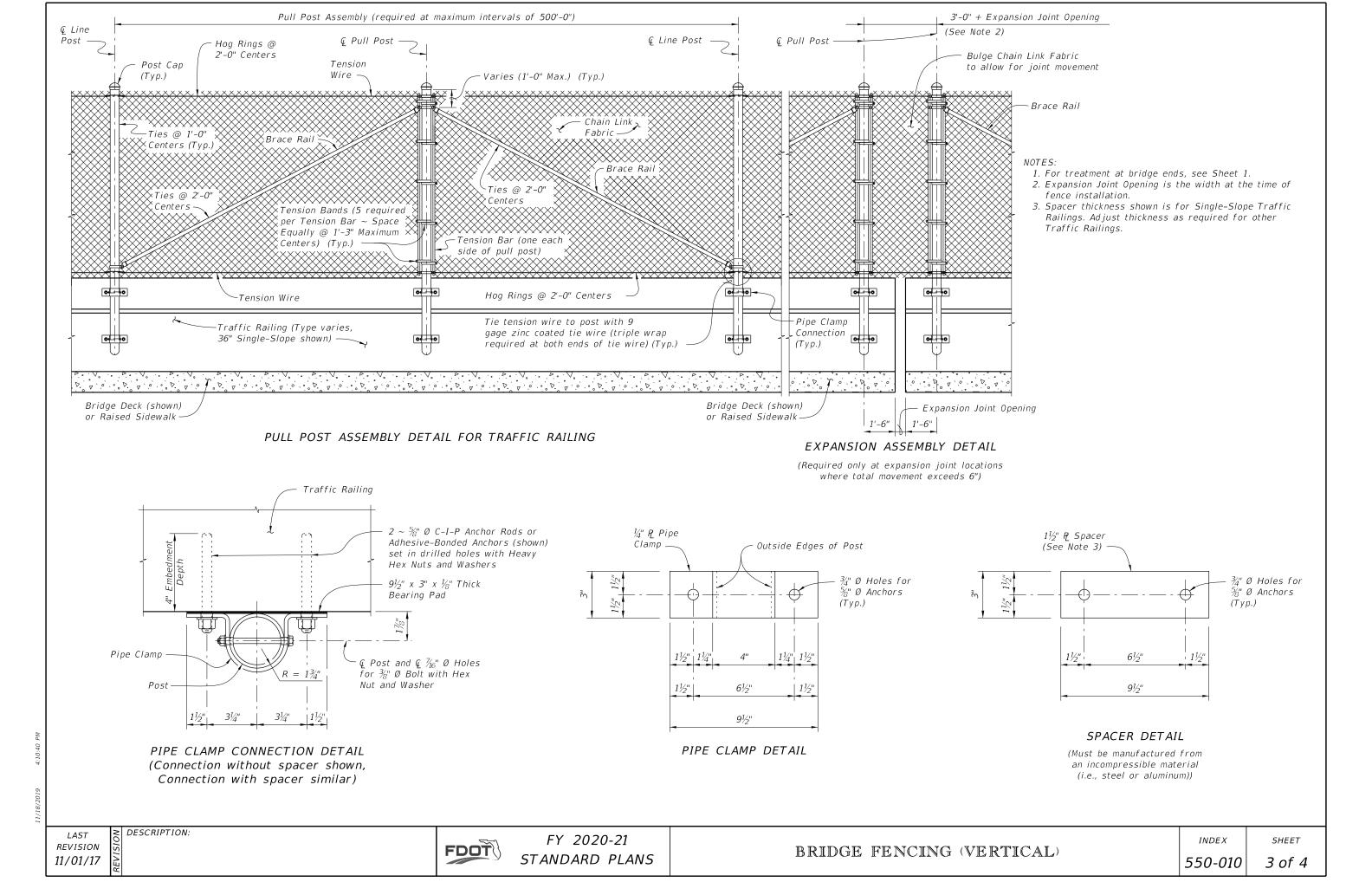
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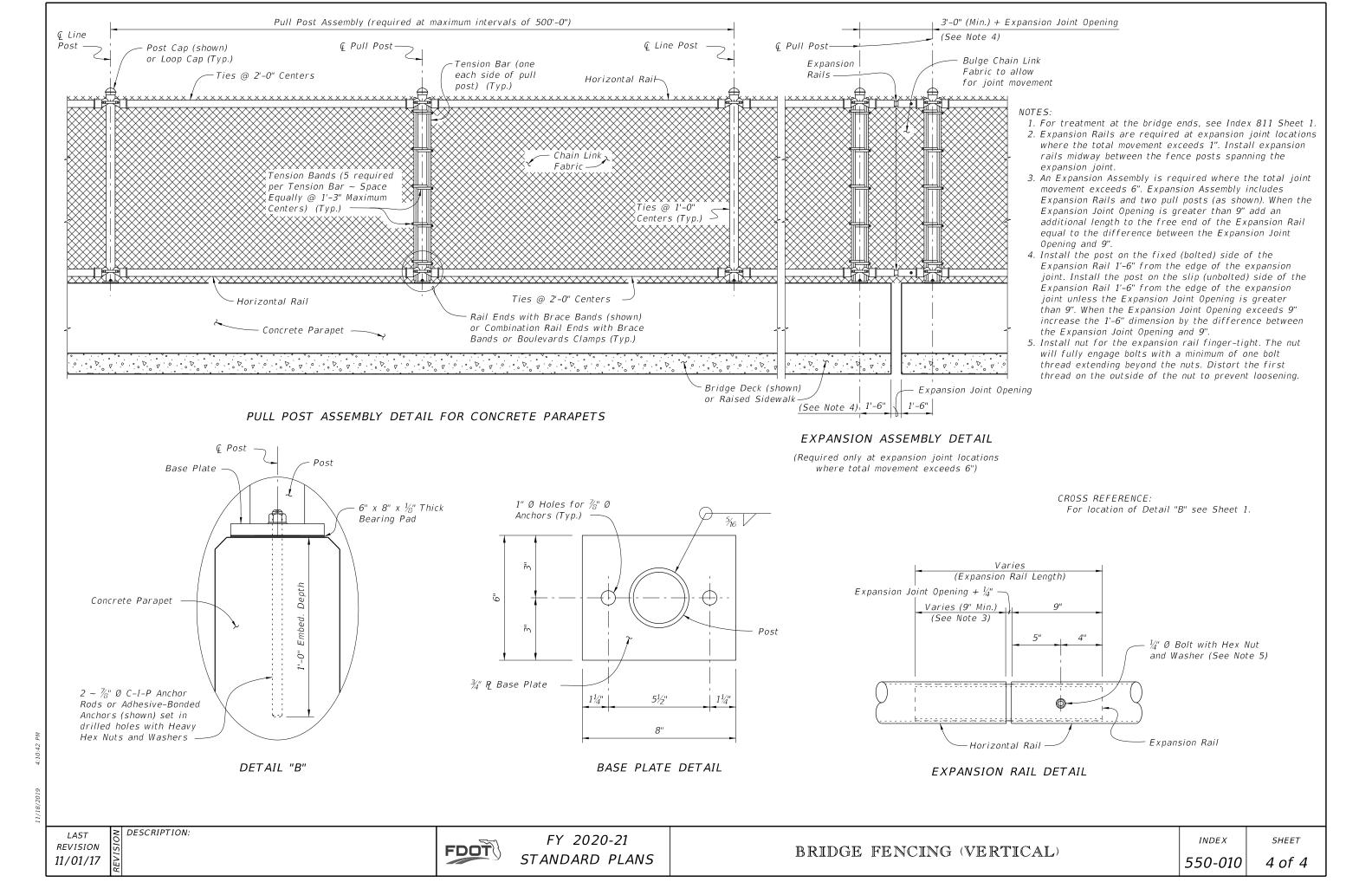
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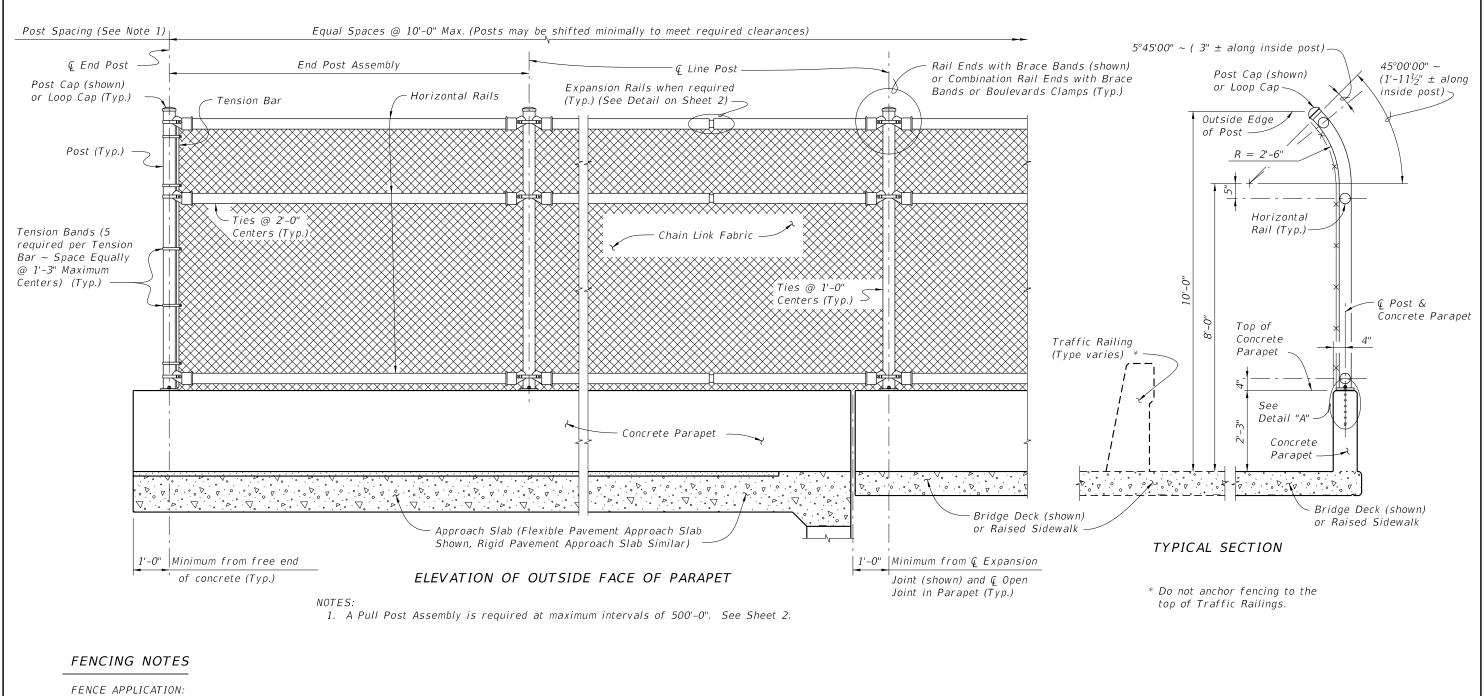
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SHEET 2 of 4







This bridge fence can only be used on sidewalk installations separated from traffic by a traffic railing.

FENCE INSTALLATION:

Install posts plumb (within a tolerance of $\pm 1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable.

See Index 521-820 - Pedestrian/Bicycle Bullet Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

DESCRIPTION:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, ties, tension bars and bands, post and loop caps, base plates, anchor rods, bolts, nuts, washers, shim plates, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components and Pull Post Assembly Detail see Sheet 2. For Table of Post Attachment Components and Detail "A" see Sheet 3.

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FDOT

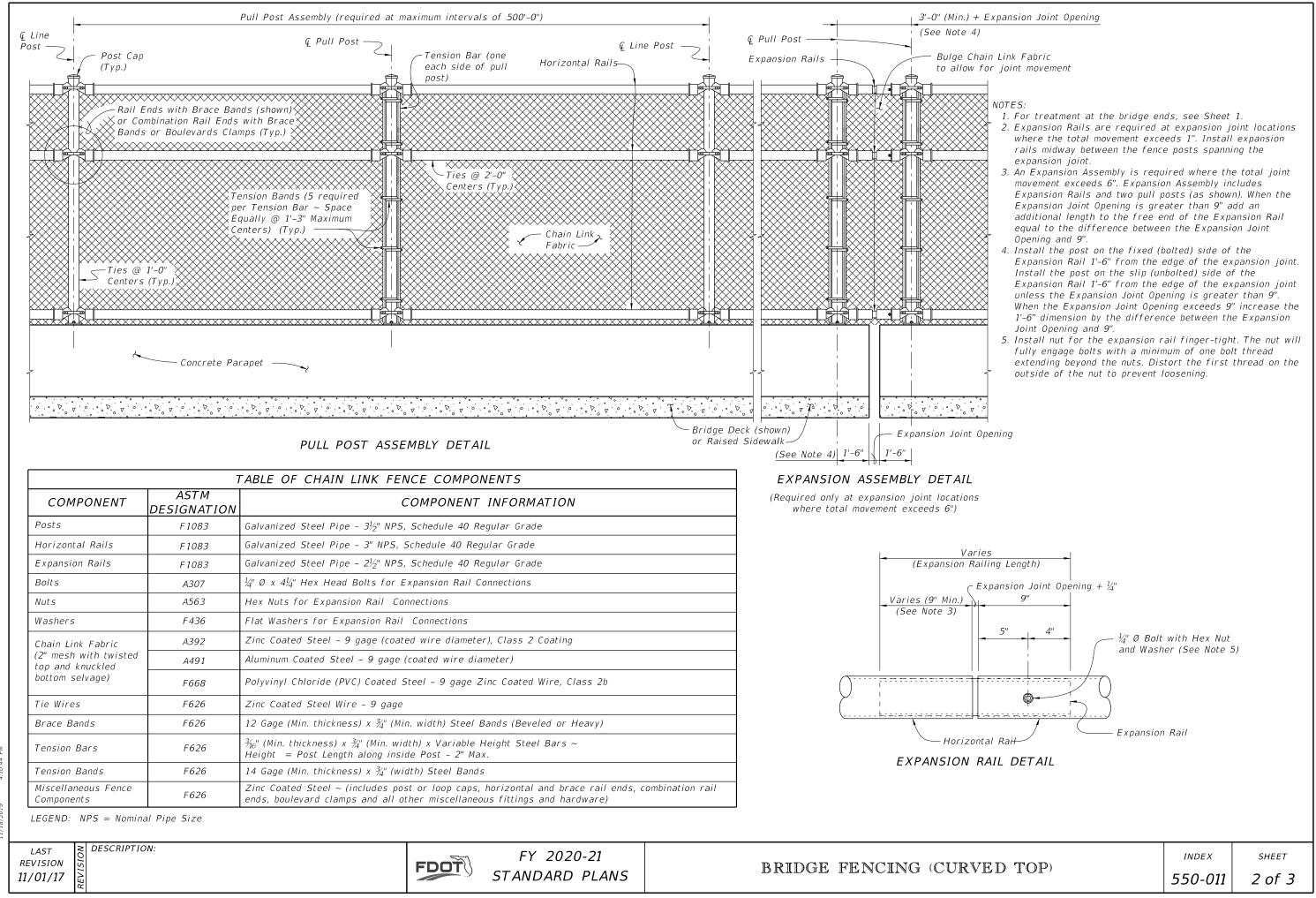
FY 2020-21 STANDARD PLANS

BRIDGE FENCING (CURVED TOP)

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0100/01/11

TABLE OF POST ATTACHMENT COMPONENTS					
COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION			
Base Plates	A36 or A709 Grade 36	¾" Steel P			
Shim Plates	A36 or A709 Grade 36 or B209 Alloy 6061–T6 or B221 Alloy 6063–T5	Plate thicknesses as required, Holes in shim plates will be $^{34}\!\!\!/^{\!\!\!/}$ Ø			
Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "			
C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "			
Nuts	A563	Hex Nuts for Base Plate Connections			
Washers	F 436	Flat Washers for Base Plate Connections			
Bearing Pads (Plain)	-	In accordance with Specification Section 932 for ancillary structures			

POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

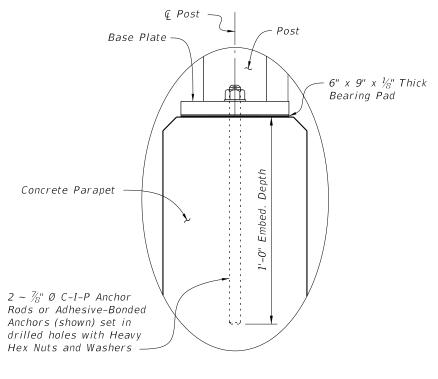
After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562.

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates and Base Plates) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

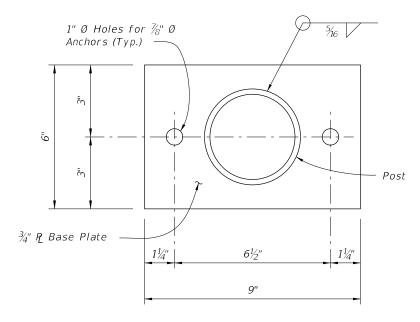
ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.



DETAIL "A"

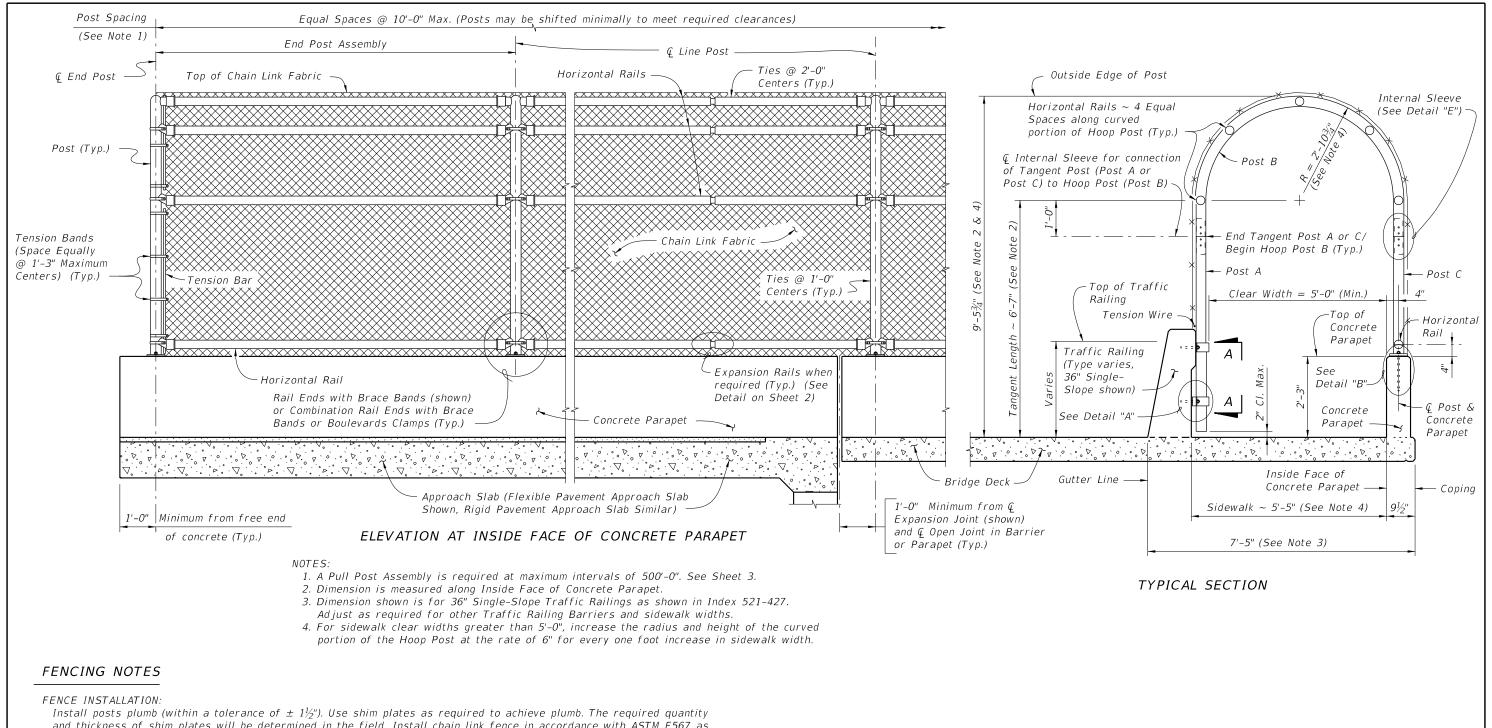


BASE PLATE DETAIL

CROSS REFERENCE:

For location of Detail "A" see Sheet 1.

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and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable.

TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

CONCRETE PARAPET DETAILS:

DESCRIPTION:

See Index 521-820 - Pedestrian/Bicycle Railing for Concrete Parapet details. Provide fencing in lieu of aluminum bullet railing as shown on Index 521-820.

LIMITS OF FENCING:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes posts, horizontal and expansion rails, brace bands, rail ends, combination rail ends, boulevard clamps, chain link fabric, tension wire, ties, hog rings, tension bars and bands, pipe clamps, base plates, anchor rods, bolts, nuts, washers, shim plates, spacers, neoprene pads, miscellaneous fence fittings and hardware and all incidental materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components and Table of Post Attachment Components see Sheet 2. For Pull Post Assembly Detail, View A-A and Detail "A" see Sheet 3. For Detail "B" and "E" see Sheet 4.

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BRIDGE FENCING (ENCLOSED)

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Posts

TABLE OF CHAIN LINK FENCE COMPONENTS

Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade

Galvanized Steel Pipe - 2½" NPS, Schedule 40 Regular Grade

Galvanized Steel Pipe - 2" NPS, Schedule 40 Regular Grade

Aluminum Coated Steel - 9 gage (coated wire diameter)

Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating

14 gage (Min. thickness) $x \frac{3}{4}$ " (Min. width) Steel Bands

clamps and all other miscellaneous fittings and hardware) $\frac{3}{8}$ " Ø x $4\frac{1}{4}$ " Hex Head Bolts for Internal Sleeve connections

 $\frac{1}{4}$ " Ø x $4\frac{1}{4}$ " Hex Head Bolts for Expansion Rail connections

Hex Nuts for Internal Sleeve and Expansion Rail connections

Flat Washers for Internal Sleeve and Expansion Rail connections

Type I (Aluminum Coated Steel Wire) - 7 gage

Zinc Coated Steel Wire - 9 gage

Zinc Coated Steel Wire - 12 gage

Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating

Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b Zinc Coated Wire

12 gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)

Height = Tangent or Hoop Length - Barrier or Parapet Height - 2" max.

Zinc Coated Steel ~ (includes horizontal rail ends, combination rail ends, boulevard

 $^{3}\!\!/_{6}$ " (Min. thickness) x $^{3}\!\!/_{4}$ " (Min. width) x Variable Height Steel Bars \sim

COMPONENT INFORMATION

ASTM

DESIGNATION

F1083

F1083

F1083

A392

A491

F668

A824 & A817

F626

F626

F626

F626

F626

F626

A307

A563

F436

- 1. Expansion Rails are required at expansion joint locations where the total movement exceeds 1". Install expansion rails midway between the fence posts spanning the expansion joint.
- 2. An Expansion Assembly is required where the total joint movement exceeds 6". Expansion Assembly includes Expansion Rails and two pull posts (see Sheet 3). When the Expansion Joint Opening is greater than 9" add an additional length to the free end of the Expansion Rail equal to the difference between the Expansion Joint Opening and 9".
- 3. Install nut for the expansion rail finger-tight. The nut will fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening.

TABLE OF POST ATTACHMENT COMPONENTS				
	COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION	
Pipe	Clamps	A36 or A709 Grade 36	¼" Steel ዊ	
Base	Plates	A36 or A709 Grade 36	¾" Steel P	
Shim	Plates	A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $\frac{3}{4}$ " Ø	
Spac	ers	-	Plate thickness varies based on Traffic Railing type. (See Detail "A")	
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Base Plate Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods \sim 7_8 " Ø x $14\frac{1}{2}$ "	
Base	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{7}{8}$ " Ø x $14\frac{1}{2}$ "	
Bolts	5	A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts	
Nuts		A563	Hex Nuts for Pipe Clamp and Base Plate Connections	
Wash	ners	F 436	Flat Washers for Pipe Clamp and Base Plate Connections	
Bear	ing Pads (Plain)	-	In accordance with Specification Section 932 for Ancillary Structures	

POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

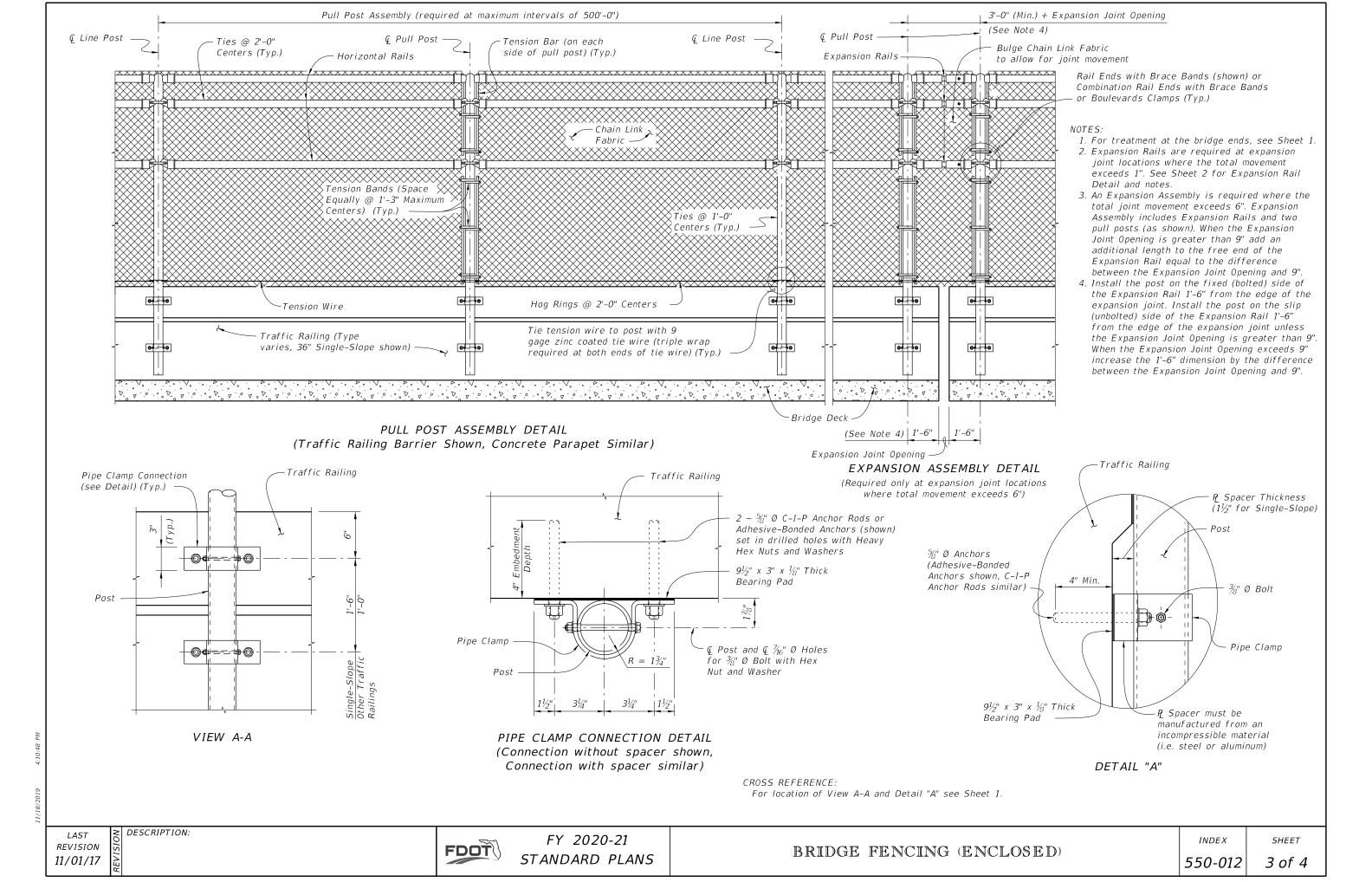
Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

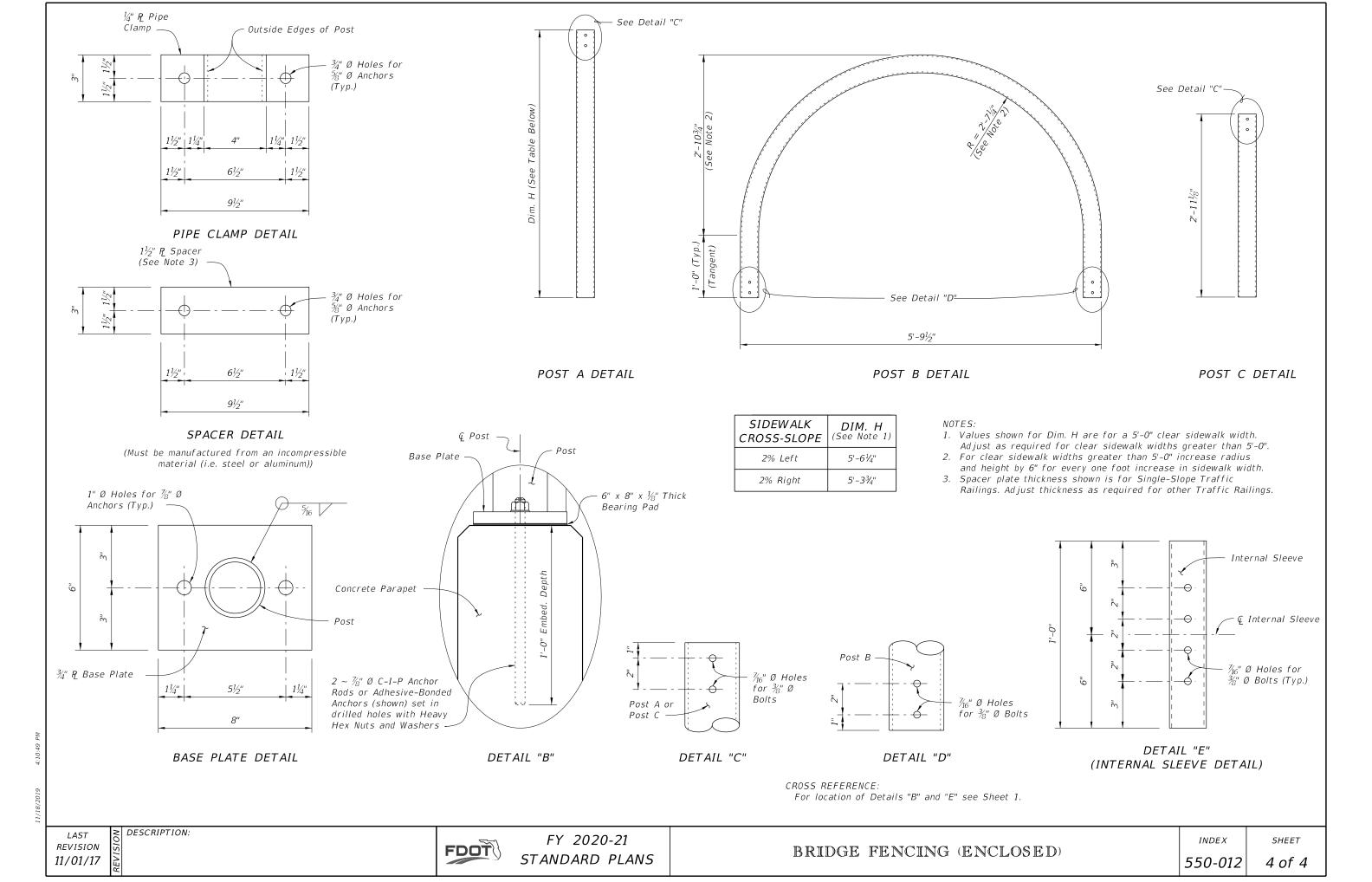
ADHESIVE-BONDED ANCHORS AND DOWELS:

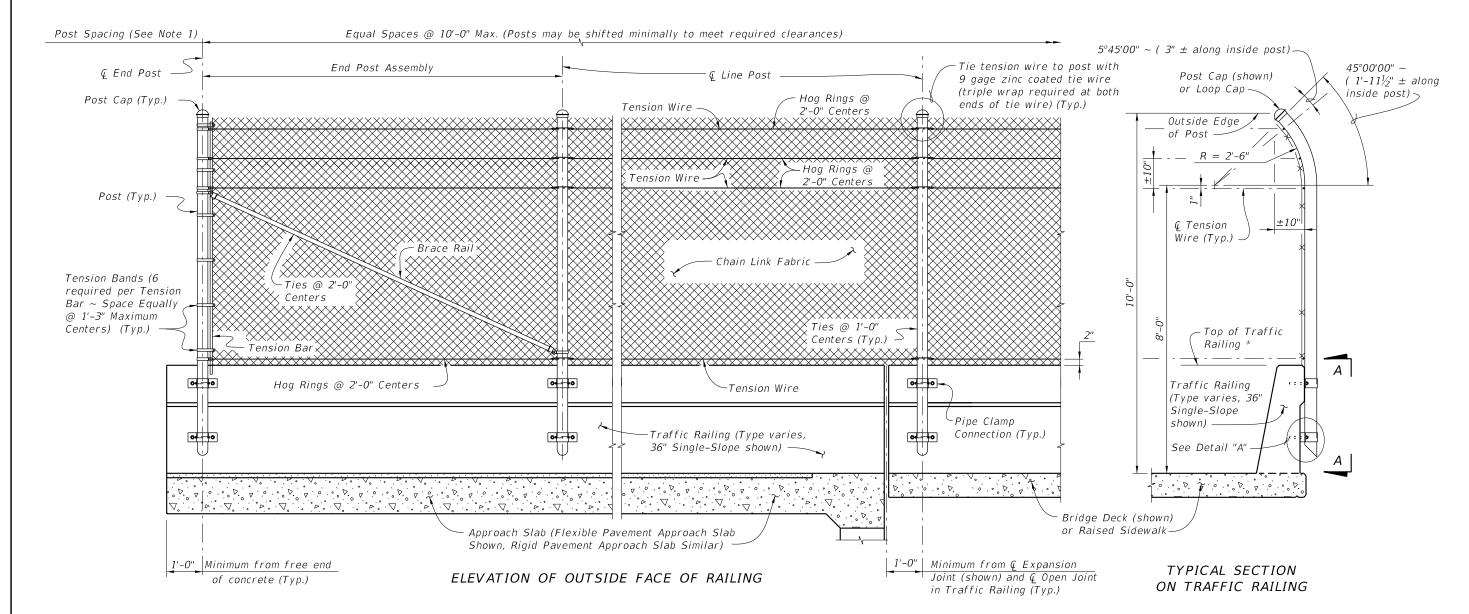
Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

DESCRIPTION:







NOTES:

1. A Pull Post Assembly is required at maximum intervals of 500'-0". See Sheet 3.

* Do not anchor Fencing to the top of Traffic Railings.

FENCING NOTES

FENCE INSTALLATION:

Install posts plumb (within a tolerance of \pm $1\frac{1}{2}$ "). Use shim plates as required to achieve plumb. The required quantity and thickness of shim plates will be determined in the field. Install chain link fence in accordance with ASTM F567 as applicable. TRAFFIC RAILING DETAILS:

See Superstructure Sheets for Traffic Railing details.

LIMITS OF FENCING:

DESCRIPTION:

Limits of fencing are from begin of approach slab at Begin Bridge to end of approach slab at End Bridge, unless otherwise shown in the plans.

PAYMENT:

Payment will be made under Fencing, Type R. Payment includes all materials and labor required to complete installation of the fence.

CROSS REFERENCE:

For Table of Fence Components, Table of Post Attachment Components, View A-A and Detail "A" see Sheet 2.

For Pull Post Assembly Detail for Traffic Railing see Sheet 3.

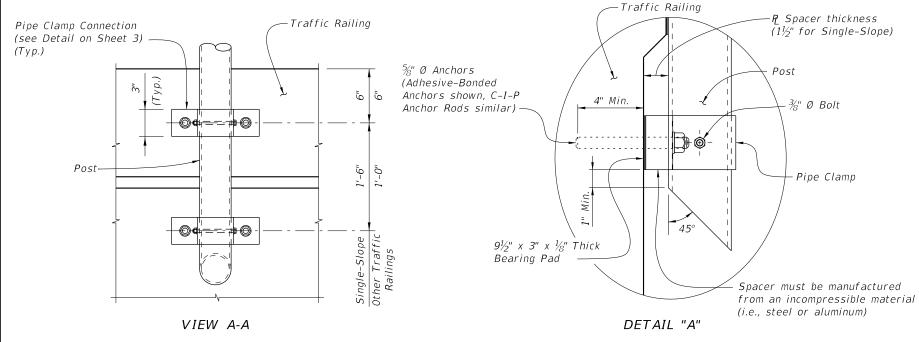
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FDOT

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TABLE OF CHAIN LINK FENCE COMPONENTS					
COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION			
Posts	F1083	Galvanized Steel Pipe - 3" NPS, Schedule 40 Regular Grade			
Chain Link Fabric (2" mesh with twisted	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating			
top and knuckled bottom selvage)	A491	Aluminum Coated Steel - 9 gage (coated wire diameter)			
	F668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Class 2b			
Tie Wires	F626	Zinc Coated Steel Wire - 9 gage			
Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)			
Tension Bars	F626	$\frac{3}{16}$ " (Min. thickness) x $\frac{3}{4}$ " (Min. width) x 6'-10" (Min. height) Steel Bars			
Tension Bands	F626	14 Gage (Min. thickness) x ¾" (Min. width) Steel Bands			
Miscellaneous Fence Components	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)			
- · · · · · · · · · · · · · · · · · · ·	A824 & A817	Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating			
Tension Wire		Type I (Aluminum Coated Steel Wire) - 7 gage			
Hog Rings	F626	Zinc Coated Steel Wire - 12 gage			
Brace Rails	F1083	Galvanized Steel Pipe – $1\frac{1}{4}$ " NPS, Schedule 40 Regular Grade			

	TABLE OF POST ATTACHMENT COMPONENTS				
	COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION		
Pipe	Clamps	A36 or A709 Grade 36	¼" Steel ዊ		
Base	Plates	A36 or A709 Grade 36	¾" Steel P		
Shim	Plates	A36 or A709 Grade 36 or B209 Alloy 6061-T6 or B221 Alloy 6063-T5	Plate thicknesses as required; Holes in shim plates will be $\frac{34}{4}$ Ø		
Space	ers	-	Plate thickness varies based on traffic railing type (See Detail "A")		
Pipe Clamp Connection	Adhesive Anchor Rods	F1554 Grade 36	Fully threaded Headless Anchor Rods $\sim \%$ " Ø x 6" (no spacer) or $\%$ " Ø x (6" + spacer thickness)		
Pipe (Conne	C-I-P Anchor Rods	F1554 Grade 36	Hex Head Anchor Rods $\sim \frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)		
Bolts		A307	¾" Ø x 4¾" Hex Head Bolts for Pipe Clamp Connections to Posts		
Nuts		A563	Hex Nuts for Pipe Clamp Connections		
Wash	ers	F436	Flat Washers for Pipe Clamp Connections		
	ing Pads n Neoprene)		In accordance with Specification Section 932 for Ancillary Structures		



POST ATTACHMENT NOTES

ANCHOR RODS, NUTS AND WASHERS:

After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. COATINGS:

Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS:

Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole installation.

WELDING:

All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required.

CROSS REFERENCE:

For location of View A-A and Detail "A" see Sheet 1.

REVISION 11/01/17

DESCRIPTION:

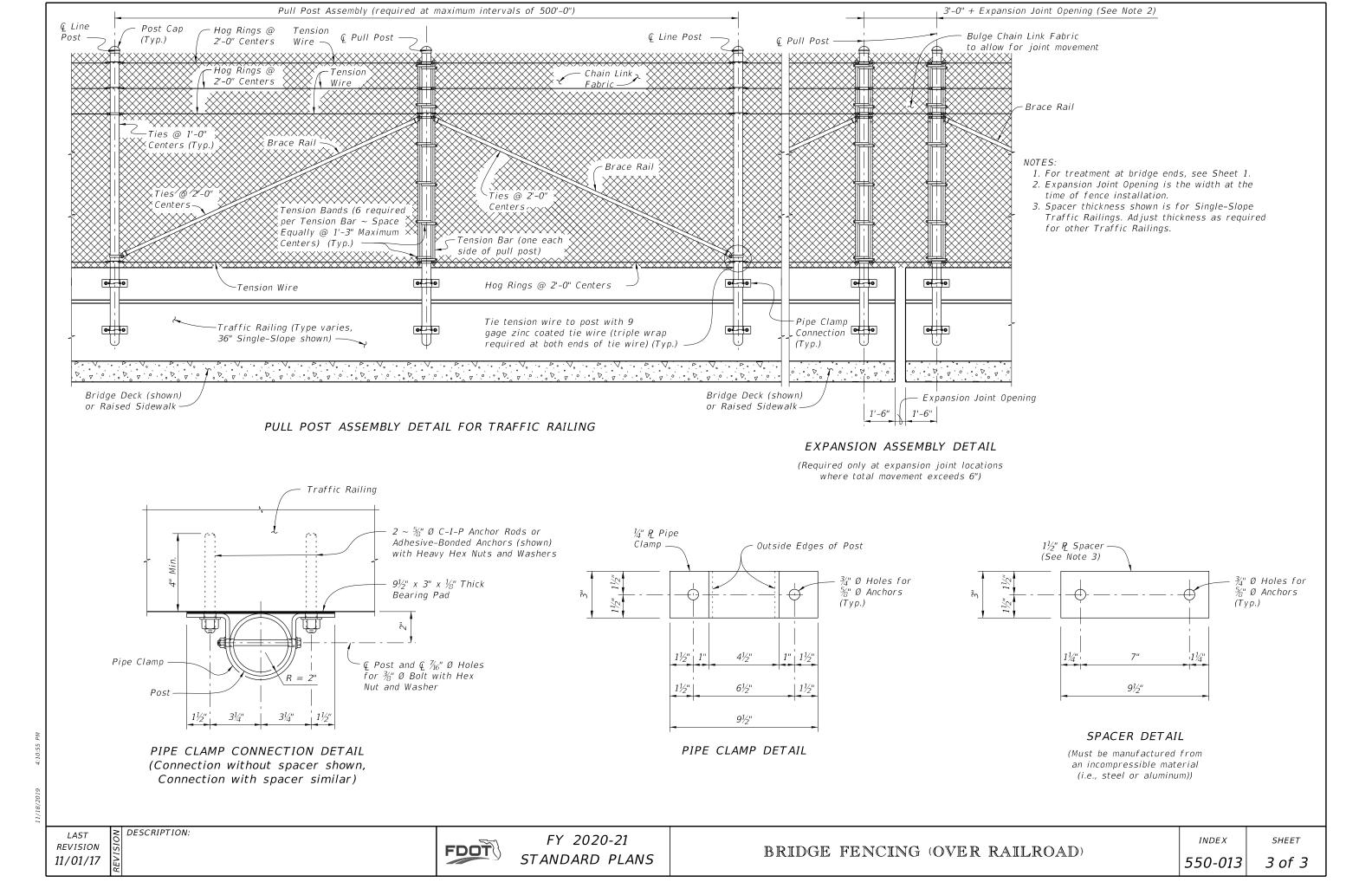
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BRIDGE FENCING (OVER RAILROAD)

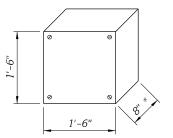
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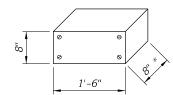


- 1. Furnish and install approved Conduits, Fittings and Embedded Junction Boxes (EBJ's) in accordance with Specification Sections 630 and 635, this Standard, the National Electric Code (NEC) and as directed by the Engineer.
- 2. Furnish and install Embedded Junction Boxes (EJB) with weatherproof covers sized in accordance with NEC requirements and the maximum size limits shown. Install EJB adjacent to the Begin and End of Bridges, Begin and End of Retaining Walls, (except omit EJB adjacent to the Bridge unless a precast Traffic Railing with junction slab is used), and at other locations as necessary to maintain 300 foot maximum spacing. See Plans for additional locations and details.
- 3. For Conduit not designated for future use, see Plans for details. For Conduit designated for future use, stub out and cap the Conduit. Drive a 3'-0" \pm long $\frac{3}{4}$ " (min.) diameter Steel Pipe flush with the ground line adjacent to the end of the Conduit as shown on Sheets 2, 3 or 4. Provide the location of the stub out with Steel Pipe to the Engineer for inclusion on the As-Built Plans.
- 4. Shift vertical Railing reinforcement symmetrically to provide 2" clearance to EJB. Space shifted vertical reinforcement at minimum 3" centers. Cut horizontal Railing reinforcement to provide 2" clearance to EJB and provide supplemental reinforcement as shown. To facilitate placement of Conduit, Expansion Fittings, and Expansion/Deflection Fittings, shift reinforcing a maximum of 1" but do not cut railing reinforcing to facilitate Conduit or Fittings. Do not bundle Conduits, or Conduit and horizontal reinforcement.

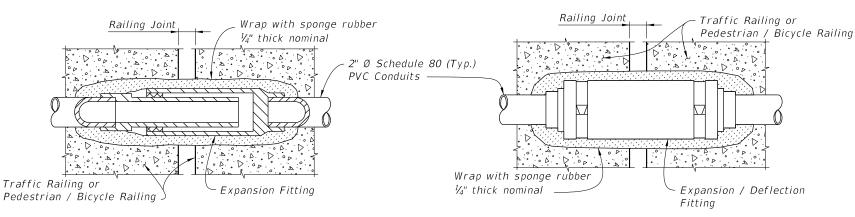
* Reduce to 6" maximum when installed in Pedestrian/ Bicycle Railings.



EJB "A" Double or Triple Conduit (Maximum Dimensions)



EJB "B" Single Conduit (Maximum Dimensions)



DETAIL "A" EXPANSION FITTING DETAIL

DETAIL "B" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / CONCRETE) DETAIL "C" EXPANSION / DEFLECTION FITTING DETAIL (CONCRETE / SOIL)

GENERAL

REVISION 11/01/18

DESCRIPTION:

FDOT

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CONDUIT DETAILS - EMBEDDED

INDEX

.Wrap' with sponge rubber

2" Ø PVC Conduits

1/3" thick nominal

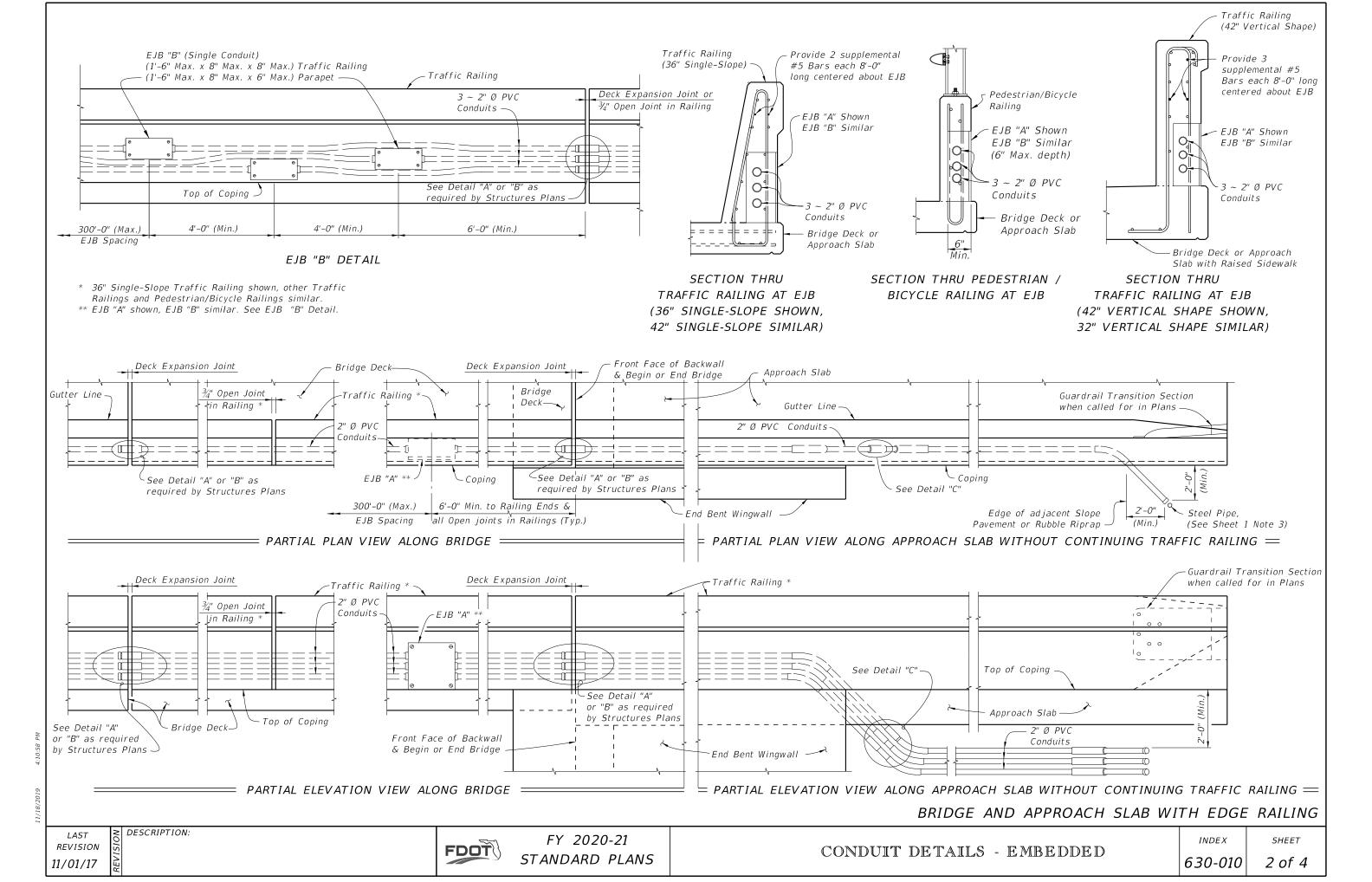
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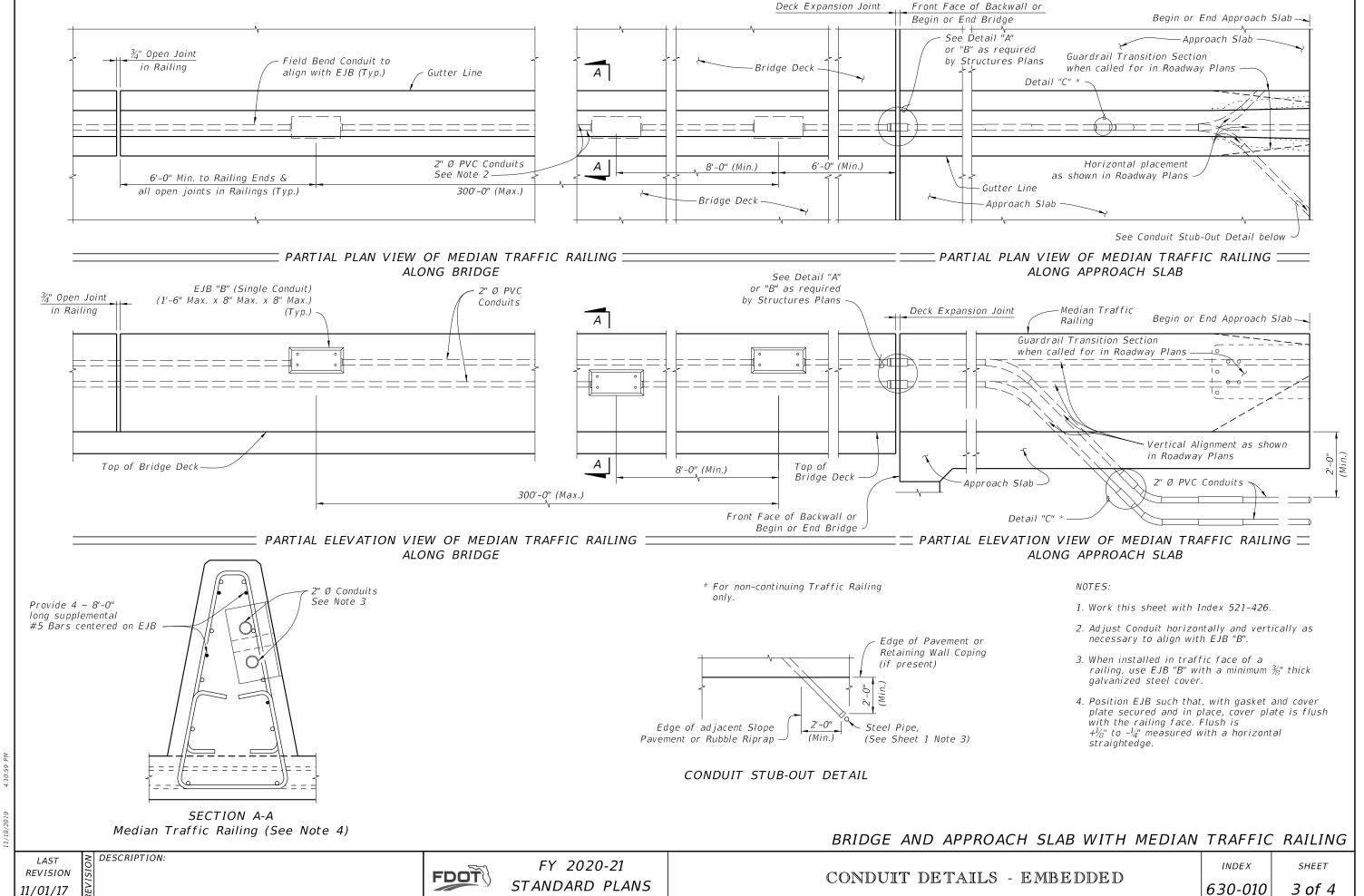
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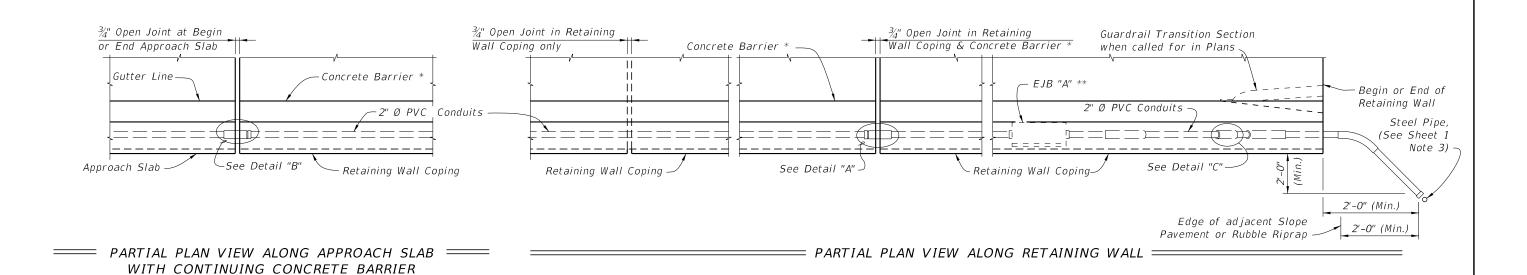
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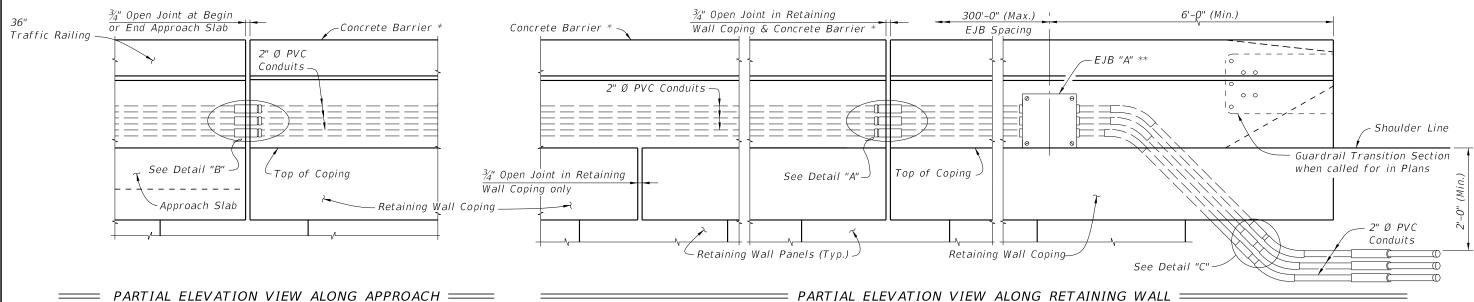
Expansion / Deflection

630-010









— PARTIAL ELEVATION VIEW ALONG APPROACH — SLAB WITH CONTINUING Concrete Barrier (Retaining Wall Mounted Concrete Barrier shown, Traffic Railing similar)

* Index 521-610 Concrete Barrier/Junction Slab shown, other railings and parapets similar.

** EJB "A" shown EJB "B" similar. See EJB "B" Detail on Sheet 2.

APPROACH SLAB AND RETAINING WALL WITH CONCRETE BARRIER

LAST REVISION 11/01/18

DESCRIPTION:

FDOT

FY 2020-21 STANDARD PLANS

CONDUIT DETAILS - EMBEDDED

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BOX GIRDER MAINTENANCE LIGHTING NOTES:

- 1. Submit shop drawings to the Engineer detailing the layout of the maintenance lighting system for the entire structure. The shop drawings must include, but not be limited to, the following items:
 - a. Conduit layout and installation details through diaphragms, around post-tensioning (PT) ducts, lateral bracing and cross frames as necessary.
 - b. Conduit access through box girder end diaphragms with minimum 1" clearance in all directions.
 - c. Conduit expansion fitting details.
 - d. Fastener details for the interior electrical system.
 - e. Single line diagram showing mini power centers, switches, contactors, timers, etc.
 - f. Mini power center details including circuit breaker details.
 - g. Mini power center mounting details if required.
 - h. Feeder schedule.
- 2. Ensure installation meets all requirements of the latest edition of the National Electrical Code (NEC) and local ordinances. Install grounding in accordance with NEC Article 250. Maintain separation between 480V and 120V Conductors / Conduits
- 3. Furnish all labor, equipment, materials, and incidentals required for a complete and functional installation.
- 4. Use only new, unused and Underwriters Laboratories (UL) listed equipment and materials for outdoor use.
- 5. Furnish and install polyvinyl chloride (PVC) conduit in conformance with UL Section 651, NEC Section 347 and NEMA TC-2, UV-resistant and schedule 80. Bend conduits as necessary to connect to loads.
- 6. Provide PVC sleeve 2" larger in diameter than conduit to accommodate construction tolerance.
- 7. Install a UL labeled expansion fitting for specified PVC conduit at all structure expansion joints. Provide certification that the expansion fitting meets the following minimum requirements: Compatibility with the connected conduits, waterproof, UV protected and allows longitudinal movement equal to that of the Expansion Joint.
- 8. Use only Alloy 316 stainless steel supporting hardware. Provide minimum $\frac{3}{6}$ of fasteners. For concrete or SIP form mounting, provide anchor bolts (expansion, drop-in or adhesive) suitable for dynamic loading (due to vibration caused by traffic). Install fasteners to avoid conflicts with reinforcing steel and PT ducts. For structural steel mounting, do not attach fasteners to main members, i.e. webs and flanges.
- 9. Furnish power distribution at 480V AC. 1 phase, with step down transformers at regular intervals. Furnish 7.5 KVA mini power center with eight 20A breakers as the step down transformer, feeding a maximum of 20 lamps and 20 receptacles. Each mini power center will provide power to no more than 1000' of bridge, preferably 500' on each side of the mini power center. 480V top feed, 120V bottom feed to maintain separation.
- 10. Furnish and install lighting contactors to switch the 480V AC feeding the mini power centers.
- 11. Furnish and install copper conductors, Type XHHW. Do not use any conductor larger than #4 AWG.
- 12. Provide enough slack in all interior cable terminations to allow for minor shifting of the structure.
- 13. Furnish and install National Electric Manufacturers Association (NEMA) Type 4X (non-metallic) surface mounted boxes sized in
- 14. Furnish and install 120V duplex receptacles (GFI, NEMA Type 5-20R), in non-metallic outlet boxes at 50' maximum on centers. Provide each receptacle with a gasketed weather-protective outdoor plate. Maximum wire size to connect to receptacles is #12 AWG.
- 15. Furnish and install surface mounted, fully enclosed, incandescent light fixtures with gasketed clear globes and wire guards at 50' maximum on centers. Provide 100 watt, 130 volt, vibration resistant and brass base incandescent lamps.
- 16. Provide six hour reset timers for each circuit to turn off the lighting system automatically.

CROSS REFERENCES:

- 1. For Maintenance Light Details, see Sheet 2.
- 2. For actual bridge section, see Structures Plans.

DESCRIPTION:

