

Effective for Projects with Lettings in the Fiscal Year (FY) from July 1, 2019 through June 30, 2020

FY 2019-20 Standard Plans for Road and Bridge Construction Topic No. 625-010-003 State of Florida Department of Transportation
Office of Design
Mail Station 32
605 Suwannee Street
Tallahassee, Florida 32399-0450

FDOT FY 2019-20 STANDARD PLANS

NOTICE

The Standard Plans are intended to support the various engineering processes for construction and maintenance operations on the State Highway System. They are established to ensure the application of uniform standards in the preparation of contract plans for construction of roadways and structures. These Standard Plans may be used for maintenance operations or adopted by other authorities for use on projects under their jurisdiction.

It is the responsibility of the Engineer of Record using these Standard Plans to determine the fitness for a particular use of each standard in the design of a project. The inappropriate use of and adherence to these standard Plans does not exempt the engineer from the professional responsibility of developing an appropriate design.

PATENTED DEVICES, MATERIALS AND PROCESSES

The use of any design, method, process, material or device either expressed or implied by these standards that are covered by patent, copyright, or proprietary privilege is the sole responsibility of the user. Any infringement on the rights of the inventor, patentee, assignee or licensee shall be the sole responsibility of the user. For additional information refer to Subsection 7–3 of the FDOT Standard Specifications for Road and Bridge Construction.

DISTRIBUTION OF EXEMPT PUBLIC DOCUMENTS:

It is the policy of the Department to protect the State Highway System's infrastructure by defining the responsibilities for disclosure and use of sensitive documents showing the structural elements used in the design and construction of Department structures. Section 119.071(3)(b), Florida Statute (F.S.), provides that these sensitive documents are exempt from Chapter 119, F.S., Florida's public records law. In accordance with Section 119.071(3)(b), F.S., the Department has adopted Procedure 050-020-026, Distribution of Exempt Public Documents Concerning Department Structures and Security System Plans, to define the method and responsibilities for disclosure and use of these sensitive documents.

Structure is defined in Section 334.03(27), F.S., as "a bridge, viaduct, tunnel, causeway, approach, ferry slip, culvert, toll plaza, gate, or other similar facility used in connection with a transportation facility" which would include related pipes and pipe systems. However, for the purpose of the public records law and Procedure 050-020-026, the Department has determined that the term "structure" includes "bridges with an opening of more than 20 feet between undercopings of abutments or spring lines of arches or extreme ends of openings for multiple boxes, and those other bridges subject to safety inspection under Section 335.074, F.S." A roadway is not otherwise a structure for the purposes of Procedure 050-020-026.

Therefore, plans, blueprints, schematic drawings, and diagrams of structures owned by the Department are exempt from the public records provisions of Chapter 119, F.S. This exemption includes draft, preliminary, and final formats as described in Procedure 050-020-026 and includes paper, electronic, and other formats. The Department has provided for the limited release of such documents in Procedure 050-020-026.

Entities or persons outside the Department requesting or receiving copies of any portion of plans or other documents considered Exempt Documents under Procedure 050-020-026 must complete and submit a request form (Form No. 050-020-26). The form also advises the requestor that the entity or person receiving the documents shall maintain their exempt status. This procedure applies to all Department internal or contracted staff who have access to such Exempt Documents in their Department work. Refer to Procedure 050-020-026 for additional requirements.

CERTIFICATION STATEMENT

I hereby certify that these Standard Plans were compiled under my responsible charge from designs prepared, examined, adopted, and implemented by the Florida Department of Transportation in accordance with established procedures, and as approved by the Federal Highway Administration.

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Approved for Use on Federal Aid Profec

James Christian, Divi

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State Structures Design Engineer Robert V. Robertson, Jr.



ABBREVIATIONS

FY 2019-20 STANDARD PLANS

Abbreviation	Meaning	Abbreviation	9-20 STANDARD PLANS Meaning	Abbreviation	Meaning
A		(G
AASHT0	American Association Of State Highway And Transportation Officials	CP	Concrete Pipe	G	Shear Modulus
AC	Alternating Current	CSIP	Cost Savings Initiative Proposal	g	Gram
Accel.	Acceleration	CSL	Cross-hole Sonic Logging	Ga.	Gauge or Gage
ACI	American Concrete Institute	СТРВ	Cement Treated Permeable Base	Galv.	Galvanized
ADA	Americans With Disabilities Act	Ctr., Ctrs.	Center	GFI	Ground Fault Interrupter
ADT	Average Daily Traffic	Cu. Ft.	Cubic Feet	GFRP	Glass Fiber Reinforced Polymer
AFAD	Automated Flagger Assistance Device	Cu. Yd., CY,	Cubic Yard	Grd.	Ground
AISC	American Institute Of Steel Construction	L)		Н
AISI	American Iron and Steel Institute	D	Depth, Distance or Diameter	Hd.	Head
Alt.	Alternate	Dia. or Ø	Diameter	H.S., HS	High Strength
Alum.	Aluminum	Dbl.	Double	HDPE	High Density Polyethylene
ANSI	American National Standards Institute	Decel.	Deceleration	Horiz.	Horizontal
AOS	Apparent Opening Size	Deg.	Degree	HP	Horsepower or H-Pile
APL	Approved Products List	Dim.	Dimension	HSHV	High Strength Horizontal Vertic
Арр.	Approach	Dist.	Distance		I
Approx.	Approximate	DMM	Domestic Mail Manual	ID, I.D.	Inside Diameter or Identification
ARTBA	American Road & Transportation Builders Association	DPI	Ditch Point Intersection	in.	Inch(es)
Asph.	Asphalt	Dt	Ditch	Inc.	Incorporated
Assem.	Assembly	DTOE	District Traffic Operations Engineer	Int.	Interior
ASTM	American Society For Testing And Materials	I	·	Inv.	Invert
ATPB	Asphalt Treated Permeable Base	e	- Superelevation Rate	ITS	Intelligent Transportation Syst
Auxil.	Auxiliary	E.P. or EOP	Edge Of Pavement		1
AWG	American Wire Gauge	EA or Ea.	Each	JCT	Junction
AWS	American Welding Society	EIA EIA	Electronic Industries Alliance	Jt.	Joint
R	3	El. or Elev.	Elevation		K
Bot.	Bottom	Embed.	Embedment	k	kip
Brkwy.	Breakaway	EPDM	Ethylene Propylene Diene Monomer	kip	1000 Pounds
b/w	Between		Equation or Equal	ksi	Kips Per Square Inch
C		Eq.		kVA	Kilovolt Ampere
CC, C to C	Center to Center	Equip.	Equipment Et Cetera (And So Forth)	K V A	
C & G	Curb And Gutter	etc. ETP			_
	Crash Cushion		Electronic Tough Pitch Example	LA	Length Limited Access
C.C.		Ex.			
CCTV	Closed-Circuit Television	Exist.	Existing	Ib or Ibs.	Pound(s)
CFR CFRP	Code of Federal Regulations	Exp.	Expansion	lb/sy	Pounds Per Square Yard Pound force
	Carbon Fiber Reinforced Polymer	Ext.	Extension -	lbf	
cfs, CFS	Cubic Feet Per Second	/	Florido Administrativo Codo	LBR	Lime rock Bearing Ratio
CIP, C.I.P. or C-I-P	Cass In Place	FAC	Florida Administrative Code	LF	Linear Foot (Feet)
CJP	Complete Joint Penetration	FC	Friction Course	Lgth.	Length
Ckt.	Circuit	Fdn.	Foundation	Long.	Longitudinally or Longitudinal
Ę.	Center Line	F.L. or F	Flow Line	LRFD	Load Resistance Factor Design
CI.	Clearance	FI.	Florida	LRS	Low-Relaxation Strand
CMP	Corrugated Metal Pipe	FDEP	Florida Department Of Environmental Protection	LS	Lump Sum
Con.	Connection	FDOT	Florida Department Of Transportation	LSD	Lump Sum per Day
Conc.	Concrete	FHWA	Federal Highway Administration	Lt.	Left
Const.	Construct or Construction	FIB	Florida-I Beam		
Cont.	Continuation or Continuous	F.S.	Florida Statutes		
Corr.	Corrugated	FS	Far Side		
Cov.	Cover	Ft.	Foot or Feet		
		FTP	Florida Traffic Plans		<u></u>

ABBREVIATIONS

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breviation	Meaning	Abbreviation	<u>Meaning</u>	Abbreviation	<u>Meaning</u>
M m	 Meter	P.E. or PE	P Professional Engineer	: St. or ST.	Street
m²	Meter Square	Pen.	Penetration	Sta.	Station
Mach.	Machine	PPB	Pier Protection Barrier	Std.	Standard
MAS	Motorist Awareness System	PPP	Polypropylene pipe	Stg.	Strong
MASH	Manual for Assessing Safety Hardware (AASHTO)	Prest.	Prestressed	St1.	Steel
Max.	Maximum	PRS	Portable Regulatory Sign	SW	Skewed Angle
MES	Mitered End Section	psf	Pounds Per Square Foot	Swk.	Sidewalk
М.Н.	Manhole or Mounting Height	PSI or psi	Pounds Per Square Inch	SYM	Symmetrical
MHW	Mean High Water	PT	Point of Tangency or Pressure Treated		Γ
Mid.	Middle	PTFE	Polytetrafluoroethylene	T or t	Thickness, Tangent Distance or Time
Mil or Mils	One-Thousandth Of An Inch	PVC	Polyvinyl Chloride	Tan	Tangent
Min.	Minimum or Minute	(Q	T&G	Tongue and Groove
Misc.	Miscellaneous	Q	Flow Volume	TCP	Traffic Control Plan(s)
MLW	Mean Low Water	Qty.	Quantity	TCZ	Traffic Control Zone
mm	Millimeter		R	Temp.	Temperature or Temporary
Mod.	Modification	R or Rad.	Radius	Theo.	Theoretical
MOT	Maintenance Of Traffic	Rt.	Right	THW or THWN	Insulation (Flame Retardant, Moisture
MPH or mph	Miles Per Hour	R/W	Right Of Way	77777 67 777777	And Heat Resistant Thermoplastic)
MUTCD	Manual On Uniform Traffic Control Devices	RC	Reverse Crown	TMA	Truck/Trailer Mounted Attenuator
N		RCP	Reinforced Concrete Pipe	TN	Ton
/ v N	Standard Penetration Number	Rd.	Road or Round	Trans.	Transition or Transverse
NA or N/A	Not Available or Not Applicable		Roadway	TTC	Temporary Traffic Control
		Rdwy.	•		
NC	Normal Crown	Rect.	Reticuline or Rectangular	TVSS	Transmit Voltage Surge Suppression
NCHRP	National Cooperative Highway Research Program	Ref.	Reference	TX	Transmit
NDCBU	Neighborhood Delivery And Collection Box Unit	Reinf.	Reinforced or Reinforcement	Тур.	Typical
NEMA	National Electrical Manufacturers Association	Req. or Reqd.	Required		J
NHW	Normal High Water	RGS	Rigid Galvanized Steel	UL	Underwriters Laboratories
No.	Number	RPM	Raised Pavement Markers	UPS	Uninterruptible Power Supply
Nom.	Nominal	R/R or RR	Railroad	USPS	United States Postal Service
NPS	Nominal Pipe Size	RSDU	Radar Speed Display Unit	Util.	Utilities
NPT	National Pipe Thread	RU	Rack Unit	UV	Ultraviolet
NS or N.S.	Near Side	RX	Receive		/
NS	Non-Structural		5	Veh.	Vehicle
NTS	Not To Scale	S or s	Speed, Spacing or Second	Vert.	Vertical
O	·	Sch.	Schedule	VPD or Vpd.	Vehicles Per Day
0.C.	On Center	SHBR	Special Height Bicycle Railing		V
0 to 0 or 0.0.	Out to Out	Shldr.	Shoulder	W	Width or Wide
0.B.G.	Optional Base Group	SHW	Seasonal High Water	WT	Weight
0D or 0.D.	Outside Diameter	SIP	Stay In Place	WWF	Welded Wire Fabric
0z.	Ounce	SP	Superpave	WWM	Welded Wire Mesh
P	·	Spa., Spcg. or Sp.	Space(ing)(s)	WWR	Welded Wire Reinforcing
Pavt.	Pavement	Spec.	Specification		Y
PBR	Pedestrian/Bicycle Railing	sq	Square	Yd.	Yard
PC	Point Of Curvature	Sq. Ft., SF, sf or S.F.	Square Foot	Yr.	Year
PCC	Plain Cement Concrete	sq. in.	Square Inch		
pcf	Pounds per Cubic Foot	Sq. Yd., SY or S.Y.	Square Yard		
PCMS	Portable Changeable Message Sign	SR	State Road		
		<i>SS</i>	Stainless Steel		

FY 2019-20 STANDARD PLANS FOR ROAD CONSTRUCTION

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Approach Sla	abs_		22624	455-124	24" Square CFRP and SS Prestressed Concrete Pile
20900	400-090	Approach Slabs (Flexible Pavement Approaches)	22630	455-130	30" Square CFRP and SS Prestressed Concrete Pile
20910	400-091	Approach Slabs (Rigid Pavement Approaches)	22654	455-154	54" Square CFRP and SS Prestressed Concrete Pile
Bridge Expai	nsion Joints		22660	455-160	60" Square CFRP and SS Prestressed Concrete Pile
21100	458-100	Strip Seal Expansion Joint			
21110	458-110	Poured Joint With Backer Rod Expansion Joint System			5

Plans Index O00-506 Changed to Index 160-001. All Sheets: Changed Title. Sheet 1: Deleted "DESIGN SPEED" table and "RADIUS OF CURVE" table; Deleted subtitle. Sheet 2: Added Concrete Pavement note to clarify shoulder slope transitions. All Sheets: Changed Title, Subtitles, and Renumbered. Sheet 1: Deleted Superelevations Rates Tabulated and Charted Values (information can be found in FDM); combined General Notes with Old Sheet 2; Deleted all callouts for "CHARTED VALUES" on Old Sheet 2. Sheet 2: Updated Subtitle. Sheet 2: Updated Subtitle. Deleted Index, Criteria information moved to New FDM Chapter 214. Construction details moved to New Indexes 522-003 or 330-001. 000-515 Deleted Index and moved information to Index 330-001. Sheet 1: "STORAGE FACILITY" Note; Changed phone number to 407-278-2727. Sheet 1: Changed Notes to remove limitations to Limited Access Facilities and Overhead work. Clarified "TRAFFIC PACING GUIDE" notes for the requirements of site specific traffic control plans. Added Note 6 to the "TRAFFIC PACING GENERAL NOTES" for short duration operations. Changed Notes 1 and 8; Added Note 9; Changed the "Crown Dripline" in the "TREE PROTECTION BARRIER-PLAN and ELEVATION" detail; Changed the "Who Open Traching" dimension; Added root pruning trenches; Changed the "Maintain Existing Grade" call out in the "TREE PROTECTION BARRIER-PLAN detail; Changed the "Crown Dripline" call out; Added Access to the "PROTECTION BARRIER FOR TREE GROUPINGS" detail: Changed Note 1 in the "TRUNK PROTECTION" detail; Added minimum requirements for barrier posts. Sheet 1: Added "REMOVAL OF EXCESS BASE MATERIAL" details from FY 2018-19 Standard Plans, Index 000-506; Updated General Notes for plain language. Deleted DESIGN NOTES. Old Sheet 3: New Sheet 2; Deleted DESIGN NOTE; Added Special Stabilized Subbase callout. Sheet 1: Updated Reference to Index 160-001 in Note 5. New Index. Previously Index 000-506; Updated Note 6 for plain language: Moved "REMOVAL OF EXCESS BASE MATERIAL" detail to Index 120-001	G					
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All Sheets: Changed Title, Subtitles, and Renumbered. Sheet 1: Deteted Superelevations Rates Tabulated and Charted Values (information can be found in FDN): combined General Notes with Old Sheet 2; Deteted all callouts for "CHARTED VALUES" on Old Sheet 2. Sheet 2: Updated Subtitle. 000-515 Deteted Index, Criteria information moved to New FDN Chapter 214. Construction details moved to New Indexes 522-003 or 330-001. 000-516 Deleted Index and moved information to Index 330-001. 102-200 Sheet 1: STORAGE FACILITY" Note; Changed phone number to 407-278-2727. 102-600 Sheet 3: Updated "LENGTH OF LANE CLOSURES" Note. Sheet 1: Changed Notes to remove limitations to Limited Access Facilities and Overhead work. Citarified "TRAFFIC PACING GUIDE" notes for the requirements of site specific traffic control plans. Added Note 6 to the "TAFFIC PACING GENERAL NOTES" for short duration operations. Changed Notes 1 and 8: Added Note 9; Changed the "Crown Dripline" in the "TREE PROTECTION BARRIER-PLAN and ELEVATION" dimension; Changed the "No Open Trenching" dimension, Added note pruning trenches; Changed the "Crown Dripline" call out; the TREE PROTECTION BARRIER-ELEVATION" detail; Changed the "Crown Dripline" call out; Added Access to the "PROTECTION BARRIER-ELEVATION" detail; Changed the "Crown Dripline" call out; Added Access to the "PROTECTION" detail; Added minimum requirements for barrier posts. Sheet 1: Added "REMOVAL OF EXCESS BASE MATERIAL" details from FY 2018-19 Standard Plans, Index 000-506; Updated General Notes for plain language. Deleted DESIGN NOTEs. Old Sheet 3: New Sheet 2: Deleted DESIGN NOTE; Added Special Stabilized Subbase callout. 120-002 Sheet 1: Updated Reference to Index 160-001 in Note 5. New Index. Previously Index 000-506; Updated Note 6 for plain language; Moved "REMOVAL OF EXCESS BASE MATERIAL" detail to Index 120-001. New Index. Content relating to Paved or Graded Driveways moved from Sheets 5 & 6 of Old Index 000-515 and 000-516. New Index. Content relating to Paved or Graded Driveways	000-510	Sheet 1: Deleted "DESIGN SPEED" table and "RADIUS OF CURVE" table; Deleted subtitle.				
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BARRIER-PLAN and ELEVATION" dimension; Changed the "No Open Trenching" dimension; Added root pruning trenches; Changed the "Maintain Existing Grade" call out in the "TREE PROTECTION BARRIER-ELEVATION" detail; Changed the "Crown Dripline" call out; Added Access to the "PROTECTION BARRIER FOR TREE GROUPINGS" detail; Changed Note 1 in the "TRUNK PROTECTION" detail; Added minimum requirements for barrier posts. Sheet 1: Added "REMOVAL OF EXCESS BASE MATERIAL" details from FY 2018-19 Standard Plans, Index 000-506; Updated General Notes for plain language. Deleted DESIGN NOTES. Old Sheet 2: Deleted Sheet (TREATED PERMEABLE BASE OPTIONS no longer supported). Old Sheet 3: New Sheet 2; Deleted DESIGN NOTE; Added Special Stabilized Subbase callout. 120-002 Sheet 1: Updated Reference to Index 160-001 in Note 5. 160-001 New Index. Previously Index 000-506; Updated Note 6 for plain language; Moved "REMOVAL OF EXCESS BASE MATERIAL" detail to Index 120-001. New Index. Content relating to Paved or Graded Driveways moved from Sheets 5 & 6 of Old Index 000-515 and 000-516. All: Updated terminology from "Turnouts" to "Driveways"; Updated notes for plain language. Sheet 2: Added Material Types And Thicknesses Table from Old Index 000-515. Updated Asphalt Thickness values for Connections; Changed 0.B.G. from type 1 to type 2. Sheet 1: Updated Note 5 for expansion joints. Sheet 3: Deleted "KEYED JOINT" Detail; Updated the "JOINT ARRANGEMENT" Detail. Sheet 4: Updated Notes, and changed outside lane standard width to 13 ft. on all illustrations. 425-040 Editorial: Added Note Jis Editorial - Note 11 450-036 Sheet 1: Corrected Note # references in "END VIEW". 450-045 Sheet 1: Corrected Note # references in "END VIEW".	102-655	Sheet 1: Changed Notes to remove limitations to Limited Access Facilities and Overhead work. Clarified "TRAFFIC PACING GUIDE" notes for the requirements of site specific traffic control				
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330-001 All: Updated terminology from "Turnouts" to "Driveways"; Updated notes for plain language. Sheet 2: Added Material Types And Thicknesses Table from Old Index 000-515. Updated Asphalt Thickness values for Connections; Changed 0.B.G. from type 1 to type 2. Sheet 1: Updated Note 5 for expansion joints. Sheet 3: Deleted "KEYED JOINT" Detail; Updated the "JOINT ARRANGEMENT" Detail. Sheet 4: Updated Notes, and changed outside lane standard width to 13 ft. on all illustrations. 425-040 Editorial: Added back deleted note on "heavy wheel loads" in GENERAL NOTES. 450-010 Sheet 1: Added Note 13; Editorial - Note 11 450-036 Sheet 1: Corrected Note # references in "END VIEW". 450-054 Sheet 1: Corrected Note # references in "END VIEW".	160-001					
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450-010 Sheet 1: Added Note 13; Editorial - Note 11 450-036 Sheet 1: Corrected Note # references in "END VIEW". 450-045 Sheet 1: Corrected Note # references in "END VIEW". 450-054 Sheet 1: Corrected Note # references in "END VIEW".	350-001	Sheet 3: Deleted "KEYED JOINT" Detail; Updated the "JOINT ARRANGEMENT" Detail.				
450-036 Sheet 1: Corrected Note # references in "END VIEW". 450-045 Sheet 1: Corrected Note # references in "END VIEW". 450-054 Sheet 1: Corrected Note # references in "END VIEW".	425-040	Editorial: Added back deleted note on "heavy wheel loads" in GENERAL NOTES.				
450-045 Sheet 1: Corrected Note # references in "END VIEW". 450-054 Sheet 1: Corrected Note # references in "END VIEW".	450-010	Sheet 1: Added Note 13; Editorial - Note 11				
450-054 Sheet 1: Corrected Note # references in "END VIEW".	450-036	Sheet 1: Corrected Note # references in "END VIEW".				
	450-045					
450-063 Sheet 1: Corrected Note # references in "END VIEW".	450-054	Sheet 1: Corrected Note # references in "END VIEW".				
	450-063	Sheet 1: Corrected Note # references in "END VIEW".				

Standard Plans Index	Description				
450-072	Sheet 1: Corrected Note # references in "END VIEW".				
450-078	Sheet 1: Corrected Note # references in "END VIEW".				
450-084	Sheet 1: Editorial, moved top insert to distance shown; Corrected Note # references in "END VIEW".				
450-096	Sheet 1: Corrected Note # references in "END VIEW".				
450-120	Sheet 1: Changed Note 7 and 9.C; Added Note 13.				
455-400	Sheet 1: Editorial, deleted extra line in "SECTION THRU BULKHEAD". Sheet 2: Deleted Section Modulus and Prestress after Losses columns from Table (added same to SPI); Added Jacking Forces to Table.				
455-440	Sheet 1: Editorial, deleted extra line in "SECTION THRU BULKHEAD". Sheet 2: Added Initial Jacking Stress to Table; Deleted Section Modulus and Prestress after Losses columns from Table (added same to SPI); Corrected Dimension A for Bars S4 thru S7.				
460-250	Editorial, VIEW A–A.				
460-252	Editorial, Deleted extra spaces in Notes.				
462-002	Added 100% acrylic aliphatic polyurethane top coating to Types 1, 4, 9, and 10 and Notes 1 & 2				
462-003	Sheet 1: Added pocket to "FILLER OUTLET DETAIL AT HORIZONTAL SURFACES".				
509-070	Sheet 3: Updated Notes and Details previously shown on Index 711-001, Sheet 12 of 14.				
515-022	Sheet 1: Editorial, Post C1.				
515-052	Sheet 1: Corrected Note 3.H Specification reference; Changed Note 3.F.a.				
515-062	Sheet 1: Corrected Note 3.H Specification reference.				
515-070	Sheet 1: Changed end hoop Note 3 to Alloy 6063-T5 to match Index 515-062.				
	Added New Sheets: New Sheet 8: Median Barrier - 56" Height Section for Barrier-Mounted Dual Sign Support Shielding. New Sheet 23: Wall Shielding Barrier- 38" Height Section - Approach and Trailing Transition. New Sheet 24: Wall Shielding Barrier - 38" Height Section - Guardrail Connection. New Sheet 25: Wall Shielding Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding.				
521-001	Revisions (By New Sheet Number): All Sheets: Updated sheet numbers and sheet references for the above additions. Sheet 1: Updated Table of Contents. Sheet 4: Added Begin/End Barrier Sta. callout point. Sheet 6,7,9,10: Added Begin/End Variable Section Width callout points. Sheet 9: Added Flowable Fill option in PLAN view; Added Note to define Flowable Fill material and NS Concrete Fill material; Replaced the stirrup with a new standardized Bar 4V3 in "SECTION B-B". Sheet 26: Updated Bar 5V2 to use one larger pin diameter for constructability; Added Bar 4V3 for use with Split and Half Section barrier. Interim, See Roadway Design Bulletin, RDB 18-06 Sheet 1: Added "GFRP - Glass Fiber Reinforced Polymer" note.				
521-002	Sheet 1: Changed Note 3. Sheets 4 & 5: Changed "Shoulder Pavement" callout to "Shoulder Pavement & Fill"				

Standard Plans Index	Description				
521-010	Redeveloped Standard New Sheet 1: Updated designs for all variations of single-slope and existing F-Shape barriers; Updated spacing of vertical and horizontal reinforcing steel, Added a minimum transverse joint spacing; Added leave-out concept for measurement; Added accommodation for welded wire reinforcing and variable barrier heights. New Sheet 2: Added detail for terminating at 56" height barrier sections; Added detail for continuing over 44" height barrier sections.				
521-422	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial, sidewalk hook bars.				
521-423	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial, "RAILING END DETAIL" and "VIEW A-A AND B-B".				
521-426	Sheet 1: Changed Barrier Delineator Note.				
521-427	Sheet 1: Changed Barrier Delineator Note.				
521-428	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial				
521-509	All: Reorganized sheets and renumbered; Updated sheet # references. Sheet 1: Added notes moved from other sheets; Added Note 6. Sheet 2: Changed reinforcing. Sheet 3: Changed reinforcing. Sheet 4: Changed reinforcing. Sheet 5: Changed Note references to new reinforcing bars.				
521-510	All: Reorganized sheets and renumbered; Updated sheet # references. Sheet 1: Added notes moved from other sheets; Added Note 6. Sheet 2: Changed reinforcing. Sheet 3: Changed reinforcing. Sheet 4: Changed reinforcing. Sheet 5: Changed Note references to new reinforcing bars.				
521-511	Sheet 1: Updated Notes. Sheet 2: Added Bar 5R3; Changed reinforcing. Sheet 3: Added Bar 5R3; Changed reinforcing.				
521-512	Sheet 1: Updated Notes. Sheet 2: Added Note 6; changed asphalt description in SECTION B-B.				
521-513	Sheet 1: Updated Notes.				
521-514	Sheet 1: Clarified Notes 1, 2, and 7; Renumbered Notes 5 and 6. Sheet 2: Editorial Sheet 4: Editorial				
521-515	Clarified Notes 1 and 2; Changed Notes 4 and 5.				
521-610	Sheet 2: Added Note 4; Renumbered remaining notes; Changed pavement reference in Typical Section.				
521-620	Sheet 1: Corrected Cross Reference; Deleted Note 12. Sheet 2: Corrected Note # references; Added Notes 7 and 8. Sheet 3: Changed Note 1 and 3. Sheet 4: Changed Title for End Transition; Corrected Note # references; Editorial, Note 4				
521-630	Sheet 2: Corrected dimension for Bar 5U1.				
521-640	Editorial: "Traffic Railing" to "Concrete Barrier".				
521-660	Sheet 1: Changed Typical Section without sidewalk to Option 1. Sheet 2: New Sheet; Added Option 2. Sheet 3: Renumbered; Changed Typical Section Title. Sheet 4: Renumbered; Added Elevation of 4H2 Bars; Changed Note 4.				

Standard Plans Index	I Decrintion				
	Sheet 1: Added Bar 4P2 as a contractor option; Changed Bar 4S placement; Moved Bar Bending Details, "REINFORCING STEEL NOTES", Estimated Quantities, and DETAIL "A" to new Sheet 2. Sheet 2: New Sheet				
522 001	Sheet 1: Deleted the 6" Min. for turnouts or curb ramps call out and 8" dimension behind the Return Curbs in the "LONGITUDINAL SECTION"; Deleted 4'-0" dimension from driveways; Changed curb ramp type in the "SIDEWALK WITH UTILITY STRIP" detail; Added example inlets to plan views Sheet 2: Deleted 8" dimension behind the Return Curbs in the "LONGITUDINAL SECTION".				
522-002	Sheet 1: Updated General Note 1. Sheet 2: Updated CR-A dimensions to match FDOT standard sidewalk widths. Sheet 3: Moved Pavement Relief Details to Sheet 6. Sheet 4: Added Sidewalk and Clarified details for CR-E. Sheet 5: Updated Notes; Clarified dimensions. Sheet 8: Added dimensions for Curb Transitions.				
522-003	New Index - Information for Concrete Flared Driveways moved from old Index 000-515.				
536-001	Sheet 1: Deleted optional conditions for washer under nuts (Notes 4 & 5); Deleted "Type II" from Table of Contents. Sheet 9: Updated Trailing Anchorage design and removed "Type II" designation; Deleted Soil Plate; Added Breakaway Post and Steel Tube Foundation at Second Post Location; Deleted Offset Block at Second Post Location; Added Two Ground Strut Supports; Changed Cable Anchor Plate to Opposite Side on Double Face Trailing Anchorage. Sheet 10: Changed the Steel Tube Foundation depth; Added new detail for ground strut (C Channel Shape). Sheet 18: Updated Trailing Anchorage drawing; Removed "Type II" designation Sheet 22: In Washer detail title, Removed "Type II" designation, Replaced with "Trailing Anchorage".				
536-002	Sheet 3: Removed Departure Line Sheet 4: Removed Crash Cushion sizing information.				
544-001	Sheet 1: Deleted Concrete Barrier and Guardrail Applications Tables; Changed the GENERAL NOTES; Updated Departure Lines, Length Restrictions, and other call outs. Sheet 2: Updated Design Length, Location Station, and other call outs.				
	Interim, See Roadway Design Bulletin, RDB 18-07 Sheet 1: Updated all details. Sheet 2: New Sheet; Added details for SHORT-TERM RAISED RUMBLE STRIPS.				
546-010	Interim, All Sheets; See Roadway Design Bulletin, RDB 18-03				
548-020	Added durability requirements for FRP reinforcing to the FDOT MSE RETAINING WALL CLASSIFICATION TABLE.				
570-010	Changed General Notes to remove Specification 162 reference.				
	Sheet 1: Changed General Notes; Changed the Under 4" and 4" and Larger Caliper Tree sizes; Changed the Stake Spacing, Anchors and Mulch callouts. Sheet 2: Changed the Under 4" and 4" and Larger Caliper Tree sizes; Changed the Stake Spacing Anchors and Mulch callouts; Changed the Palm Planting Note; Changed the Min. Wood Braces callout for the Palm Planting on Slope.				
630-001	All Sheets: Reorganized; Updated Notes. Sheet 2: Deleted FIGURE A "Pullbox Entry Of Conduit Under Sidewalks".				
	Sheet 1: Clarified that EJB "A" is for double or triple conduit. Sheet 2: Corrected callout detailing so arrows pointed to EJB's correctly. Sheet 4: Changed Traffic Railing to Concrete Barrier.				
634-002	Cleaned up, Reorganized, and Changed Notes.				
635-001	Updated Notes; Added 6" Min. Depth to Ground Rod from top of Pull and Fiber Optic Boxes.				

Standard Plans Index	I DACCEINTION					
649-010	Sheet 1: Note 5B Added "including plate washers". Sheet 2: ELEVATION, Deleted minimum threaded length for 'BC'; PLAN view, Deleted "Size And" from #11 bar description; Deleted "MAXIMUM ALLOWABLE MOMENT" column from table; Changed table "STEEL STRAIN POLE DATA TABLE".					
649-020	Sheet 1: Changed Note 2; Note 3.E Deleted "ASTM F2329 galvanizing and added "ASTM A36" pl washers; Note 3.J added "including plate washers"; Changed Note 4. Sheet 3: ELEVATION corrected longitudinal bar callout, added reference to Table; Added cross reference to Tables on Sheet 2.					
649-030	Added DS/25/5.0 to DRILLED SHAFT Table; Updated values of bolts, BA and BC values in POLE, BASE PLATE and ARM CONNECTION Table.					
649-031	Sheet 1: Changed Notes 4.D.a, 5, and 6.B. Sheet 3: Clarified ARM SPLICE length; Clarified SECTION D-D Inside Bend Radius.					
654-001	New Index; Moved details for Mid-Block Crossing RRFB signs from Index 700-120.					
659-010	Updated Notes, "SIGN MOUNTING DETAIL", and "DETAIL OF OPPOSING SIGNS SPAN WIRE MOUNTED"; Deleted "ADJUSTABLE HANGER FOR SIGN MOUNTING" detail.					
660-001	All Sheets: Reorganized; Clarified Notes.					
665-001	Sheet 1: Added (See DETAIL "A") to the Concrete Pedestal and Strain Poles; Changed the Pushbutton distance to the edge of concrete; Changed Note 2; Deleted back-to-back pushbutton mounts in DETAIL "A".					
676-010	Updated Notes; Reorganized Sheet; Added optional conduit to "POLE MOUNTED CONTROLLER CABINET- CONCRETE POLE" detail.					
700-010	Sheet 1: Clarified Example Notes. Sheet 2: Changed title (lower right); Sheet 3: Clarified "OFFSET SIGN" Notes and * INSTALLING FRANGIBLE COLUMN SUPPORTS Notes Changed Wall Thk for 8" OD column. Sheet 4: Clarified NOTES 1, 2.B, 3.A. Added galvanized steel to 3.A.2.c; Changed 8" post thickness and weld dimensions. Sheet 5: Added U-bolt to PLAN view and Max. column 0.D. to ELEVATION view. Sheet 6: Deleted "WIND BEAM PLACEMENT DETAILS"; Changed Wind Beam Placement Notes; Changed "SECTION A-A" to "VIEW A-A"; Changed top cantilever dimension.					
700-011	Sheet 1: Changed Note 2; Added Note 6; Changed "SECTION C-C" callouts; Corrected Bolt Spa. dimension lines and Min. sign panel length in Sign Detail; Added break lines in column and foundation in TYPICAL SECTION; Added NPS designation for column pipes to Table. Sheet 2: Added Class I Concrete for "BASE AND FOUNDATION DETAIL"; Added break lines to "STUB DETAIL".					
700-012	Sheet 1: Corrected Note 3.D.b; Changed Note 3.C.					
700-013	Sheet 1: Corrected Note 3.C.b; Table 1 changed NPS callout style.					
700-020	Sheet 1: Changed spacing of three columns; Clarified Note 2.A and B; Changed Note 3.B.c. Sheet 2: Corrected weld symbols; Clarified column sections are steel. Sheet 3: Clarified Wind Beam Tables; Added nylon washer note; Changed % sign depth Wind Beam spacing.					
700-030	Changed Title; Deleted the 12'-0' Max - Depth of Truss in the SIDE ELEVATION; Changed Washers (changed lock to std); Changed spacing of Hangers and Wind Beams to match 700-020 changes; Changed the WIND BEAMS AND VERTICAL HANGERS Table; Deleted Max. chord spacing from SIDE ELEVATION.					
700-040	Sheet 2: Corrected callout for longitudinal bars (FC to FL) in PLAN and ELEVATION of DRILLED SHAFT.					
700-041	Sheet 1: Changed Note 4.C.a					
700-102	Sheet 8: Corrected text positioning. Sheet 10: Deleted MOT-2-06 and MOT-3-06. Sheet 11: Updated due to deleted signs on Sheet 10.					

Standard Plans Index	l Decrintion			
700-103	Deleted Index. Criteria located to FDM 230.			
700-109	Changed "OBJECT MARKER DETAIL" to show two Wind Beams. Changed Notes; Deleted redundant material information; Changed "DEAD END" sign requirements.			
700-110	Changed Index Title: Changed Notes; Changed bolt callouts on "SECTION A-A (Side Elevation)";			
700-120	All: Redeveloped and Renamed Index; Added Alpha-Numeric Designation system; Clarified use of Conventional and Solar power option for all assembly types.			
706-001	Sheet 4: Changed and Deleted RPMs in the "RPM PLACEMENT AT ISLANDS", Details "G" and "H". Sheet 5: Added new sheet showing the placement of raised pavement markers at limited access crossovers. Sheet 6: Added new sheet showing the placement of blue raised pavement markers.			
711-001	All: Renumbered sheets. Sheet 1: Added Route Shield details; Added "PAVEMENT MESSAGE SPACING TABLE"; Added GENERAL NOTES; Updated Pavement Message Notes. Sheet 7: Added Note 3. Sheet 8: Deleted "100" max." for Right Turn Lane Drop Details. Sheet 9: Updated Pavement Message spacing distance to S. Sheet 10: Changed Intersection Details to Standard Crosswalk Details. Moved Note 3 to Sheet 7. Sheet 11: Updated Pavement Message spacing distance to S. Sheet 12: Deleted Sheet. Information included on Index 509-070.			
715-002	Sheet 1: Changed GENERAL NOTE 4.B and Note 5.C. Sheet 2: Updated all details - deleted or revised pole dimensions. Sheet 3: Added dual dimensions to "ARM CONNECTION DETAIL" and "SECTION A-A". Deleted "ARM TABLE" and its Notes; Changed "ARM TUBE EXTRUSIONS NOTES". Sheet 4: Changed FOUNDATION Depth Requirement; Added dual dimensions to "POLE BASE ELEVATION"; Deleted All Table and Added new tables; Updated NOTES. Sheet 5: Added dual dimensions to the "BASE PLATE PLAN"; Deleted the "POLE TABLE"; Updated NOTES.			
Sheet 1: Changed Notes 2 and 4. Sheet 2: ELEVATION - Editorial; Changed "POLE DESIGN TABLE" - Deleted Column, Editor PLATE AND BOLTS DESIGN TABLE" - Changed some Base Plate Thicknesses. Sheet 3: "SECTION E-E" Changed Inside Bend Radius details.				

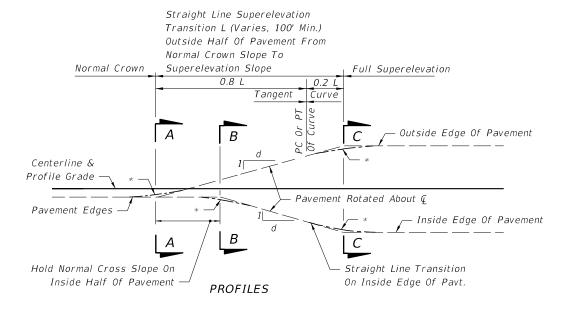
Standard Plans Index	Description				
450-063	Sheet 1: Corrected Note # references in "END VIEW".				
450-072	Sheet 1: Corrected Note # references in "END VIEW".				
450-078	Sheet 1: Corrected Note # references in "END VIEW".				
450-084	Sheet 1: Editorial, moved top insert to distance shown; Corrected Note # references in "END VIEW".				
450-096	Sheet 1: Corrected Note # references in "END VIEW".				
450-120	Sheet 1: Changed Note 7 and 9.C; Added Note 13.				
455-400	Sheet 1: Editorial, deleted extra line in "SECTION THRU BULKHEAD". Sheet 2: Deleted Section Modulus and Prestress after Losses columns from Table (added same to SPI); Added Jacking Forces to Table.				
455-440	Sheet 1: Editorial, deleted extra line in "SECTION THRU BULKHEAD". Sheet 2: Added Initial Jacking Stress to Table; Deleted Section Modulus and Prestress after Losses columns from Table (added same to SPI); Corrected Dimension A for Bars S4 thru S7.				
460-250	Editorial, VIEW A-A.				
460-252	Editorial, Deleted extra spaces in Notes.				
462-002	Added 100% acrylic aliphatic polyurethane top coating to Types 1, 4, 9, and 10 and Notes 1 & 2				
462-003	Sheet 1: Added pocket to "FILLER OUTLET DETAIL AT HORIZONTAL SURFACES".				
509-070	Sheet 3: Updated Notes and Details previously shown on Index 711-001, Sheet 12 of 14.				
515-022	Sheet 1: Editorial, Post C1.				
515-052	Sheet 1: Corrected Note 3.H Specification reference; Changed Note 3.F.a.				
515-062	Sheet 1: Corrected Note 3.H Specification reference.				
515-070	Sheet 1: Changed end hoop Note 3 to Alloy 6063-T5 to match Index 515-062.				
	Added New Sheets: New Sheet 8: Median Barrier - 56" Height Section for Barrier-Mounted Dual Sign Support Shielding. New Sheet 23: Wall Shielding Barrier- 38" Height Section - Approach and Trailing Transition. New Sheet 24: Wall Shielding Barrier - 38" Height Section - Guardrail Connection. New Sheet 25: Wall Shielding Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding.				
521-001	Revisions (By New Sheet Number): All Sheets: Updated sheet numbers and sheet references for the above additions. Sheet 1: Updated Table of Contents. Sheet 4: Added Begin/End Barrier Sta. callout point. Sheet 6,7,9,10: Added Begin/End Variable Section Width callout points. Sheet 9: Added Flowable Fill option in PLAN view; Added Note to define Flowable Fill material and NS Concrete Fill material; Replaced the stirrup with a new standardized Bar 4V3 in "SECTION B-B". Sheet 26: Updated Bar 5V2 to use one larger pin diameter for constructability; Added Bar 4V3 for use with Split and Half Section barrier. Interim, See Roadway Design Bulletin, RDB 18-06 Sheet 1: Added "GFRP - Glass Fiber Reinforced Polymer" note.				
521-002	Sheet 1: Changed Note 3. Sheets 4 & 5: Changed "Shoulder Pavement" callout to "Shoulder Pavement & Fill"				

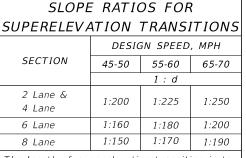
Standard Plans Index	Description				
521-010	Redeveloped Standard New Sheet 1: Updated designs for all variations of single-slope and existing F-Shape barriers; Updated spacing of vertical and horizontal reinforcing steel, Added a minimum transverse joint spacing; Added leave-out concept for measurement; Added accommodation for welded wire reinforcing and variable barrier heights. New Sheet 2: Added detail for terminating at 56" height barrier sections; Added detail for continuing over 44" height barrier sections.				
521-422	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial, sidewalk hook bars.				
521-423	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial, "RAILING END DETAIL" and "VIEW A-A AND B-B".				
521-426	Sheet 1: Changed Barrier Delineator Note.				
521-427	Sheet 1: Changed Barrier Delineator Note.				
521-428	Sheet 1: Changed Barrier Delineator Note. Sheet 2: Editorial				
521-509	All: Reorganized sheets and renumbered; Updated sheet # references. Sheet 1: Added notes moved from other sheets; Added Note 6. Sheet 2: Changed reinforcing. Sheet 3: Changed reinforcing. Sheet 4: Changed reinforcing. Sheet 5: Changed Note references to new reinforcing bars.				
521-510	All: Reorganized sheets and renumbered; Updated sheet # references. Sheet 1: Added notes moved from other sheets; Added Note 6. Sheet 2: Changed reinforcing. Sheet 3: Changed reinforcing. Sheet 4: Changed reinforcing. Sheet 5: Changed Note references to new reinforcing bars.				
521-511	Sheet 1: Updated Notes. Sheet 2: Added Bar 5R3; Changed reinforcing. Sheet 3: Added Bar 5R3; Changed reinforcing.				
521-512	Sheet 1: Updated Notes. Sheet 2: Added Note 6; changed asphalt description in SECTION B-B.				
521-513	Sheet 1: Updated Notes.				
521-514	Sheet 1: Clarified Notes 1, 2, and 7; Renumbered Notes 5 and 6. Sheet 2: Editorial Sheet 4: Editorial				
521-515	Clarified Notes 1 and 2; Changed Notes 4 and 5.				
521-610	Sheet 2: Added Note 4; Renumbered remaining notes; Changed pavement reference in Typical Section.				
521-620	Sheet 1: Corrected Cross Reference; Deleted Note 12. Sheet 2: Corrected Note # references; Added Notes 7 and 8. Sheet 3: Changed Note 1 and 3. Sheet 4: Changed Title for End Transition; Corrected Note # references; Editorial, Note 4				
521-630	Sheet 2: Corrected dimension for Bar 5U1.				
521-640	Editorial: "Traffic Railing" to "Concrete Barrier".				
521-660	Sheet 1: Changed Typical Section without sidewalk to Option 1. Sheet 2: New Sheet; Added Option 2. Sheet 3: Renumbered; Changed Typical Section Title. Sheet 4: Renumbered; Added Elevation of 4H2 Bars; Changed Note 4.				

Standard Plans Index	Sheet 1: Added Bar 4P2 as a contractor option; Changed Bar 4S placement; Moved Bar Bending				
F22 001	Sheet 1: Deleted the 6" Min. for turnouts or curb ramps call out and 8" dimension behind the Return Curbs in the "LONGITUDINAL SECTION"; Deleted 4'-0" dimension from driveways; Changed curb ramp type in the "SIDEWALK WITH UTILITY STRIP" detail; Added example inlets to plan views. Sheet 2: Deleted 8" dimension behind the Return Curbs in the "LONGITUDINAL SECTION".				
522-002	Sheet 1: Updated General Note 1. Sheet 2: Updated CR-A dimensions to match FDOT standard sidewalk widths. Sheet 3: Moved Pavement Relief Details to Sheet 6. Sheet 4: Added Sidewalk and Clarified details for CR-E. Sheet 5: Updated Notes; Clarified dimensions. Sheet 8: Added dimensions for Curb Transitions.				
522-003	New Index - Information for Concrete Flared Driveways moved from old Index 000-515.				
	Sheet 1: Deleted optional conditions for washer under nuts (Notes 4 & 5); Deleted "Type II" from Table of Contents. Sheet 9: Updated Trailing Anchorage design and removed "Type II" designation; Deleted Soil Plate; Added Breakaway Post and Steel Tube Foundation at Second Post Location; Deleted Offset Block at Second Post Location; Added Two Ground Strut Supports; Changed Cable Anchor Plate to Opposite Side on Double Face Trailing Anchorage. Sheet 10: Changed the Steel Tube Foundation depth; Added new detail for ground strut (C Channel Shape). Sheet 18: Updated Trailing Anchorage drawing; Removed "Type II" designation Sheet 22: In Washer detail title, Removed "Type II" designation, Replaced with "Trailing Anchorage".				
536-002	Sheet 3: Removed Departure Line Sheet 4: Removed Crash Cushion sizing information.				
	Sheet 1: Deleted Concrete Barrier and Guardrail Applications Tables; Changed the GENERAL NOTES; Updated Departure Lines, Length Restrictions, and other call outs. Sheet 2: Updated Design Length, Location Station, and other call outs.				
546-001	Interim, See Roadway Design Bulletin, RDB 18-07 Sheet 1: Updated all details. Sheet 2: New Sheet; Added details for SHORT-TERM RAISED RUMBLE STRIPS.				
546-010	Interim, All Sheets; See Roadway Design Bulletin, RDB 18-03				
548-020	Added durability requirements for FRP reinforcing to the FDOT MSE RETAINING WALL CLASSIFICATION TABLE.				
570-010	Changed General Notes to remove Specification 162 reference.				
580-001	Sheet 1: Changed General Notes; Changed the Under 4" and 4" and Larger Caliper Tree sizes; Changed the Stake Spacing, Anchors and Mulch callouts. Sheet 2: Changed the Under 4" and 4" and Larger Caliper Tree sizes; Changed the Stake Spacing, Anchors and Mulch callouts; Changed the Palm Planting Note; Changed the Min. Wood Braces callout for the Palm Planting on Slope.				
630-001	All Sheets: Reorganized; Updated Notes. Sheet 2: Deleted FIGURE A "Pullbox Entry Of Conduit Under Sidewalks".				
630-010	Sheet 1: Clarified that EJB "A" is for double or triple conduit. Sheet 2: Corrected callout detailing so arrows pointed to EJB's correctly. Sheet 4: Changed Traffic Railing to Concrete Barrier.				
634-002	Cleaned up, Reorganized, and Changed Notes.				
635-001	Updated Notes; Added 6" Min. Depth to Ground Rod from top of Pull and Fiber Optic Boxes.				

Standard Plans Index	x Description				
649-010	Sheet 1: Note 5B Added "including plate washers". Sheet 2: ELEVATION, Deleted minimum threaded length for 'BC'; PLAN view, Deleted "Size And" from #11 bar description; Deleted "MAXIMUM ALLOWABLE MOMENT" column from table; Changed table "STEEL STRAIN POLE DATA TABLE".				
649-020	Sheet 1: Changed Note 2; Note 3.E Deleted "ASTM F2329 galvanizing and added "ASTM A36" plate washers; Note 3.J added "including plate washers"; Changed Note 4. Sheet 3: ELEVATION corrected longitudinal bar callout, added reference to Table; Added cross reference to Tables on Sheet 2.				
649-030	Added DS/25/5.0 to DRILLED SHAFT Table; Updated values of bolts, BA and BC values in POLE, BASE PLATE and ARM CONNECTION Table.				
649-031	Sheet 1: Changed Notes 4.D.a, 5, and 6.B. Sheet 3: Clarified ARM SPLICE length; Clarified SECTION D-D Inside Bend Radius.				
654-001	New Index; Moved details for Mid-Block Crossing RRFB signs from Index 700-120.				
659-010	Updated Notes, "SIGN MOUNTING DETAIL", and "DETAIL OF OPPOSING SIGNS SPAN WIRE MOUNTED"; Deleted "ADJUSTABLE HANGER FOR SIGN MOUNTING" detail.				
660-001	All Sheets: Reorganized; Clarified Notes.				
665-001	Sheet 1: Added (See DETAIL "A") to the Concrete Pedestal and Strain Poles; Changed the Pushbutton distance to the edge of concrete; Changed Note 2; Deleted back-to-back pushbutton mounts in DETAIL "A".				
676-010	Updated Notes; Reorganized Sheet; Added optional conduit to "POLE MOUNTED CONTROLLER CABINET- CONCRETE POLE" detail.				
	Sheet 1: Clarified Example Notes. Sheet 2: Changed title (lower right); Sheet 3: Clarified "OFFSET SIGN" Notes and * INSTALLING FRANGIBLE COLUMN SUPPORTS Notes; Changed Wall Thk for 8" OD column. Sheet 4: Clarified NOTES 1, 2.B, 3.A. Added galvanized steel to 3.A.2.c; Changed 8" post thickness and weld dimensions. Sheet 5: Added U-bolt to PLAN view and Max. column 0.D. to ELEVATION view. Sheet 6: Deleted "WIND BEAM PLACEMENT DETAILS"; Changed Wind Beam Placement Notes; Changed "SECTION A-A" to "VIEW A-A"; Changed top cantilever dimension.				
	Sheet 1: Changed Note 2; Added Note 6; Changed "SECTION C-C" callouts; Corrected Bolt Spa. dimension lines and Min. sign panel length in Sign Detail; Added break lines in column and foundation in TYPICAL SECTION; Added NPS designation for column pipes to Table. Sheet 2: Added Class I Concrete for "BASE AND FOUNDATION DETAIL"; Added break lines to "STUB DETAIL".				
700-012	Sheet 1: Corrected Note 3.D.b; Changed Note 3.C.				
700-013	Sheet 1: Corrected Note 3.C.b; Table 1 changed NPS callout style.				
	Sheet 1: Changed spacing of three columns; Clarified Note 2.A and B; Changed Note 3.B.c. Sheet 2: Corrected weld symbols; Clarified column sections are steel. Sheet 3: Clarified Wind Beam Tables; Added nylon washer note; Changed % sign depth Wind Beam spacing.				
700-030	Changed Title; Deleted the 12'-0' Max - Depth of Truss in the SIDE ELEVATION; Changed Washers (changed lock to std); Changed spacing of Hangers and Wind Beams to match 700-020 changes; Changed the WIND BEAMS AND VERTICAL HANGERS Table; Deleted Max. chord spacing from SIDE ELEVATION.				
700-040	Sheet 2: Corrected callout for longitudinal bars (FC to FL) in PLAN and ELEVATION of DRILLED SHAFT.				
700-041	Sheet 1: Changed Note 4.C.a				
700-102	Sheet 8: Corrected text positioning. Sheet 10: Deleted MOT-2-06 and MOT-3-06. Sheet 11: Updated due to deleted signs on Sheet 10.				

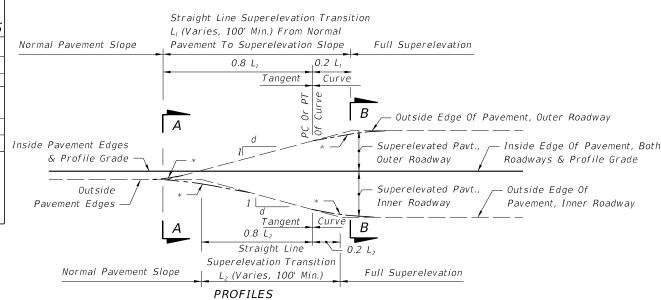
Standard Plans Index	Description			
700-103	Deleted Index. Criteria located to FDM 230.			
700-109	Changed "OBJECT MARKER DETAIL" to show two Wind Beams. Changed Notes; Deleted redundant material information; Changed "DEAD END" sign requirements.			
700-110	Changed Index Title: Changed Notes; Changed bolt callouts on "SECTION A-A (Side Elevation)";			
700-120	All: Redeveloped and Renamed Index; Added Alpha-Numeric Designation system; Clarified use of Conventional and Solar power option for all assembly types.			
706-001	Sheet 4: Changed and Deleted RPMs in the "RPM PLACEMENT AT ISLANDS", Details "G" and "H". Sheet 5: Added new sheet showing the placement of raised pavement markers at limited access crossovers. Sheet 6: Added new sheet showing the placement of blue raised pavement markers.			
711-001	All: Renumbered sheets. Sheet 1: Added Route Shield details; Added "PAVEMENT MESSAGE SPACING TABLE"; Added GENERAL NOTES; Updated Pavement Message Notes. Sheet 7: Added Note 3. Sheet 8: Deleted "100" max." for Right Turn Lane Drop Details. Sheet 9: Updated Pavement Message spacing distance to S. Sheet 10: Changed Intersection Details to Standard Crosswalk Details. Moved Note 3 to Sheet 7. Sheet 11: Updated Pavement Message spacing distance to S. Sheet 12: Deleted Sheet. Information included on Index 509-070.			
715-002	Sheet 1: Changed GENERAL NOTE 4.B and Note 5.C. Sheet 2: Updated all details - deleted or revised pole dimensions. Sheet 3: Added dual dimensions to "ARM CONNECTION DETAIL" and "SECTION A-A". Deleted "ARM TABLE" and its Notes; Changed "ARM TUBE EXTRUSIONS NOTES". Sheet 4: Changed FOUNDATION Depth Requirement; Added dual dimensions to "POLE BASE ELEVATION"; Deleted All Table and Added new tables; Updated NOTES. Sheet 5: Added dual dimensions to the "BASE PLATE PLAN"; Deleted the "POLE TABLE"; Updated NOTES.			
715-010	Sheet 1: Changed Notes 2 and 4. Sheet 2: ELEVATION - Editorial; Changed "POLE DESIGN TABLE" - Deleted Column, Editorial; "BASE PLATE AND BOLTS DESIGN TABLE" - Changed some Base Plate Thicknesses. Sheet 3: "SECTION E-E" Changed Inside Bend Radius details.			

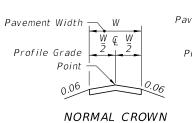




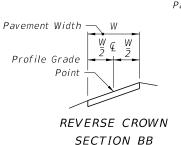
The length of superelevation transition is to be determined by the relative slope between the travel way edge of pavement and the profile grade, except that the minimum length of transition shall be 100 ft.

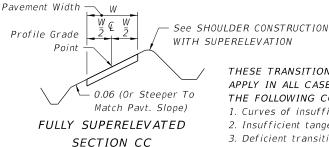
* Short Vertical Curves Are To Be Used On Construction To Avoid Angular Breaks In Edge Profiles





SECTION AA





THESE TRANSITION DETAILS ARE TO APPLY IN ALL CASES, EXCEPT UNDER THE FOLLOWING CONDITIONS:

- 1. Curves of insufficient length.
- 2. Insufficient tangent length between curves.
- 3. Deficient transition distance between a curve and other control point(s).
- 4. At PCC's or PRC's (Runoff rates are applicable).

Profile Grade Points — Pavement Width Median Slope As Indicated On Plans

NORMAL SECTION

SECTION AA

Profile Grade Points 0.06 (Or Steeper To Match Pavt. Slope) 0.05 (Or Steeper To Match Pavt. Slope) See SHOULDER CONSTRUCTION WITH SUPERELEVATION

FULLY SUPERELEVATED SECTION BB

See SHOULDER CONSTRUCTION

WITH SUPERELEVATION

2-LANE, 4-LANE OR 6-LANE PAVEMENT, NO MEDIAN

Transitions for these exceptions are to be as detailed in the plans.

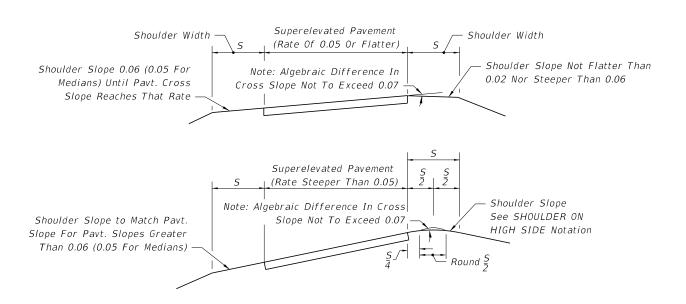
4-LANE OR 6-LANE PAVEMENT WITH MEDIAN

SUPERELEVATION TRANSITIONS

NOTES:

DESCRIPTION:

- 1. These details apply to both paved and grassed shoulders. For median shoulders use 0.05 in lieu of 0.06.
- 2. SHOULDER ON HIGH SIDE: A shoulder slope of 0.06 downward from the edge of travel way will be maintained until a 0.07 break in slope at the pavement edge is reached due to superelevation of the pavement. As the pavement superelevation increases, the 0.07 break in slope will be maintained and the shoulder flattened until the shoulder slope reaches the minimum of 0.02 downward from the edge of travel way. Any further increase in pavement superelevation will necessitate sloping the inside half of the shoulder toward the travel way and the outer half outward, both at 0.02 for superelevations 0.06-0.09 and both at 0.03 for superelevation 0.10. For shoulders with paved widths 5 feet or less see Special Shoulder Break Over Details on Sheet 2 of 2.
- 3. SHOULDER ON LOW SIDE: Maintain 0.06 cross slope across shoulder until pavement cross slope reaches 0.06. For pavement cross slopes greater than 0.06, shoulder to have same slope as pavement. See SHOULDER SLOPES ON SUPERELEVATION SECTION (Sheet 2).



SHOULDER CONSTRUCTION WITH SUPERELEVATION

REVISION 11/01/18

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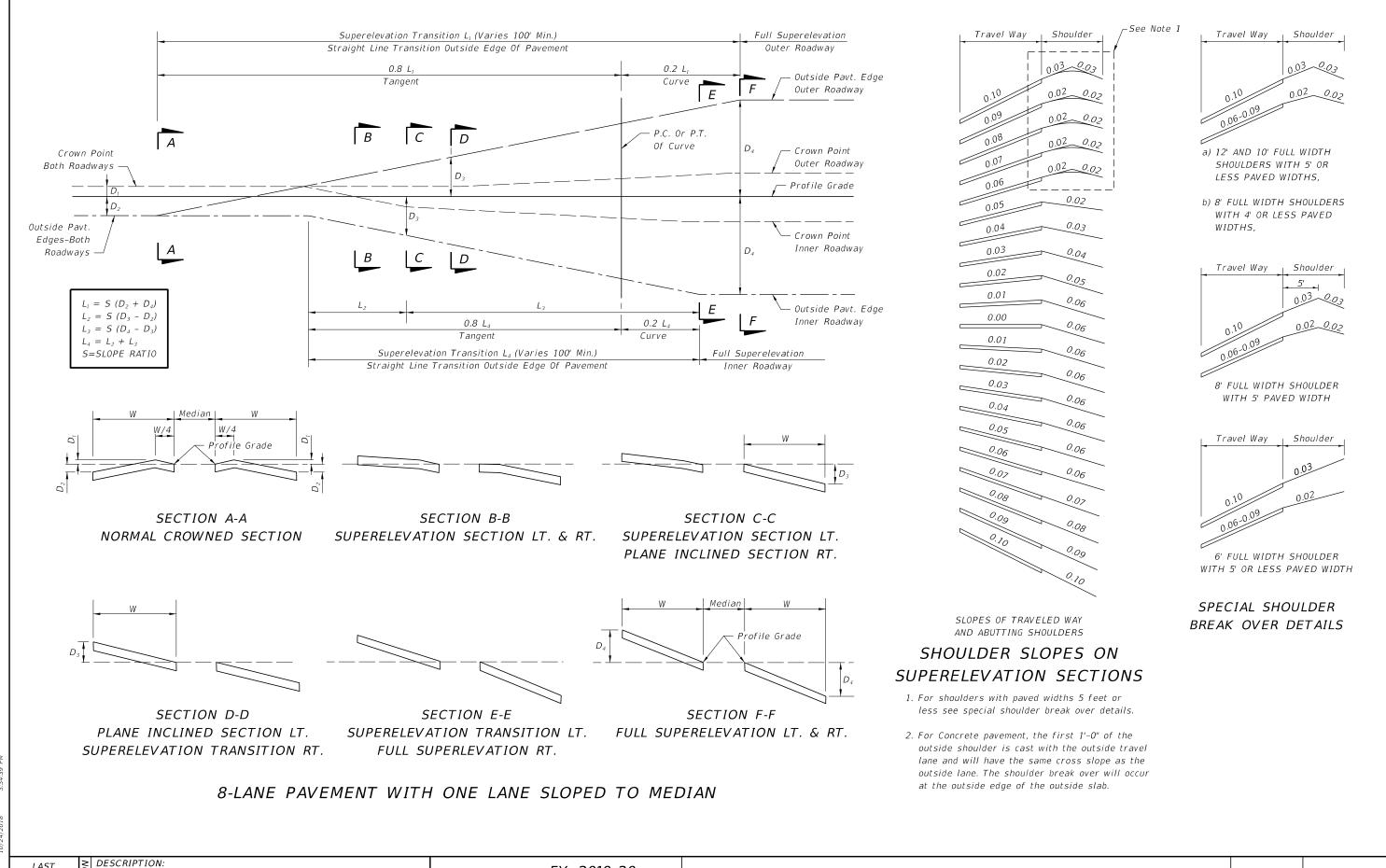
FY 2019-20 STANDARD PLANS

SUPERELEVATION TRANSITIONS -HIGH SPEED ROADWAYS

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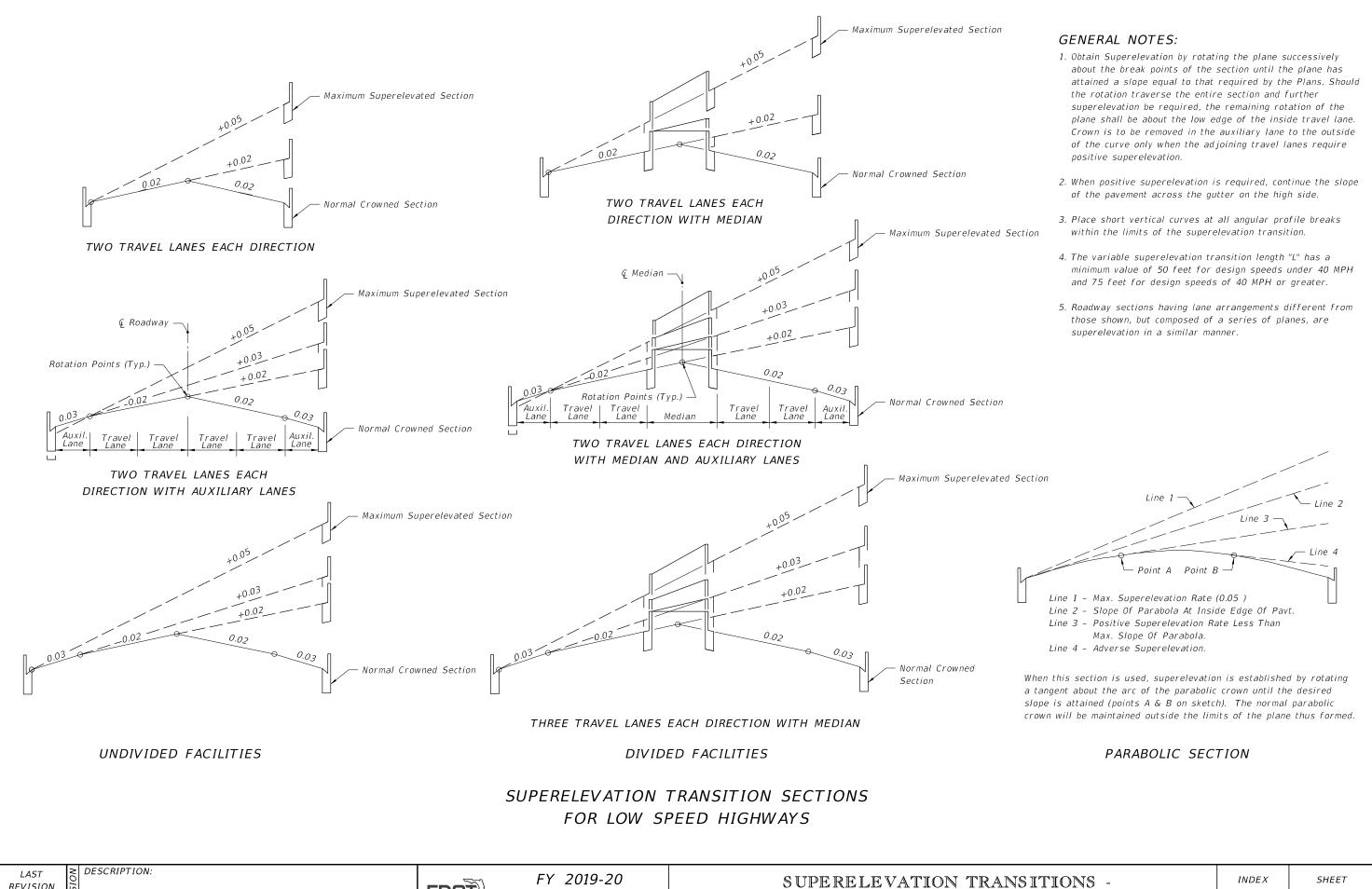
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10/24/2018 3:34:

LAST REVISION 11/01/18

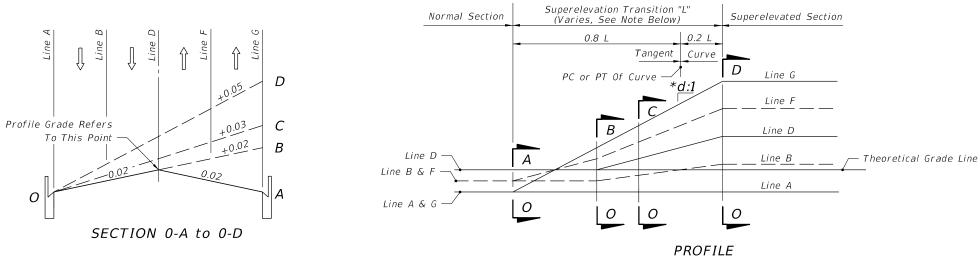
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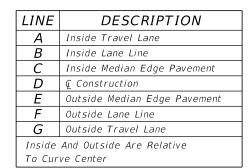


REVISION 11/01/18

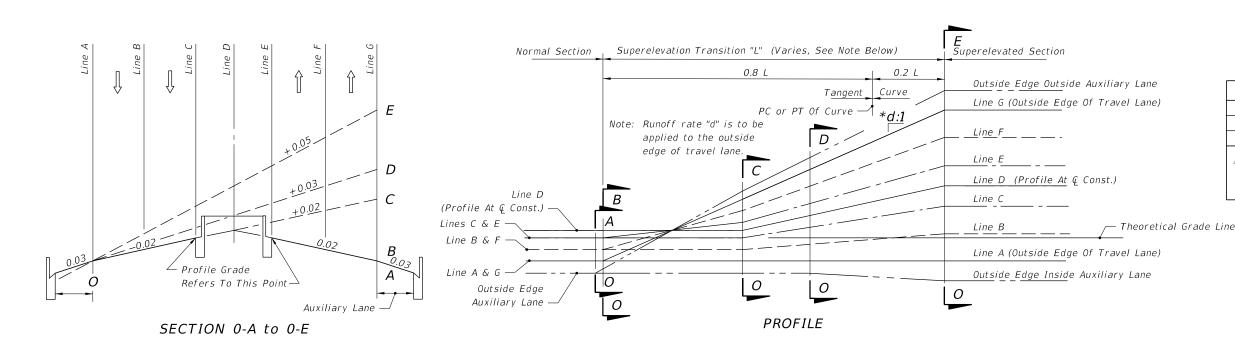
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000-511





TWO LANES EACH DIRECTION



45-50 MPH ∧ 1: 150 △ 1: 125 May Be Used For

30 MPH

40 MPH

45 MPH Under Restricted Conditions.

*d (Slope Ratio)

1: 100

1: 125

TWO LANES EACH DIRECTION WITH MEDIAN AND AUXILIARY LANE

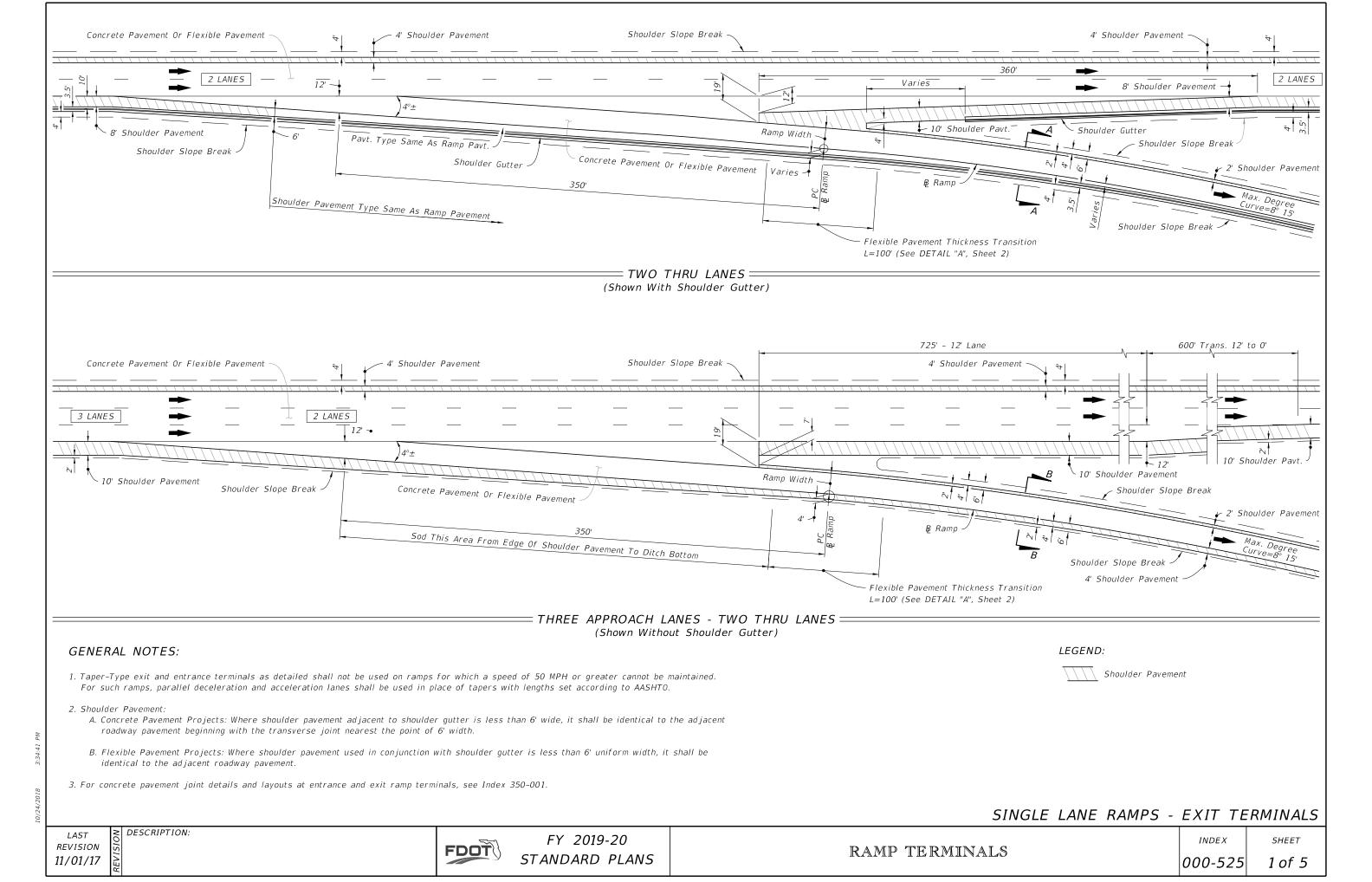
The sections and profiles shown are examples of superelevation transitions. Similar schemes should be used for roadways having other sections.

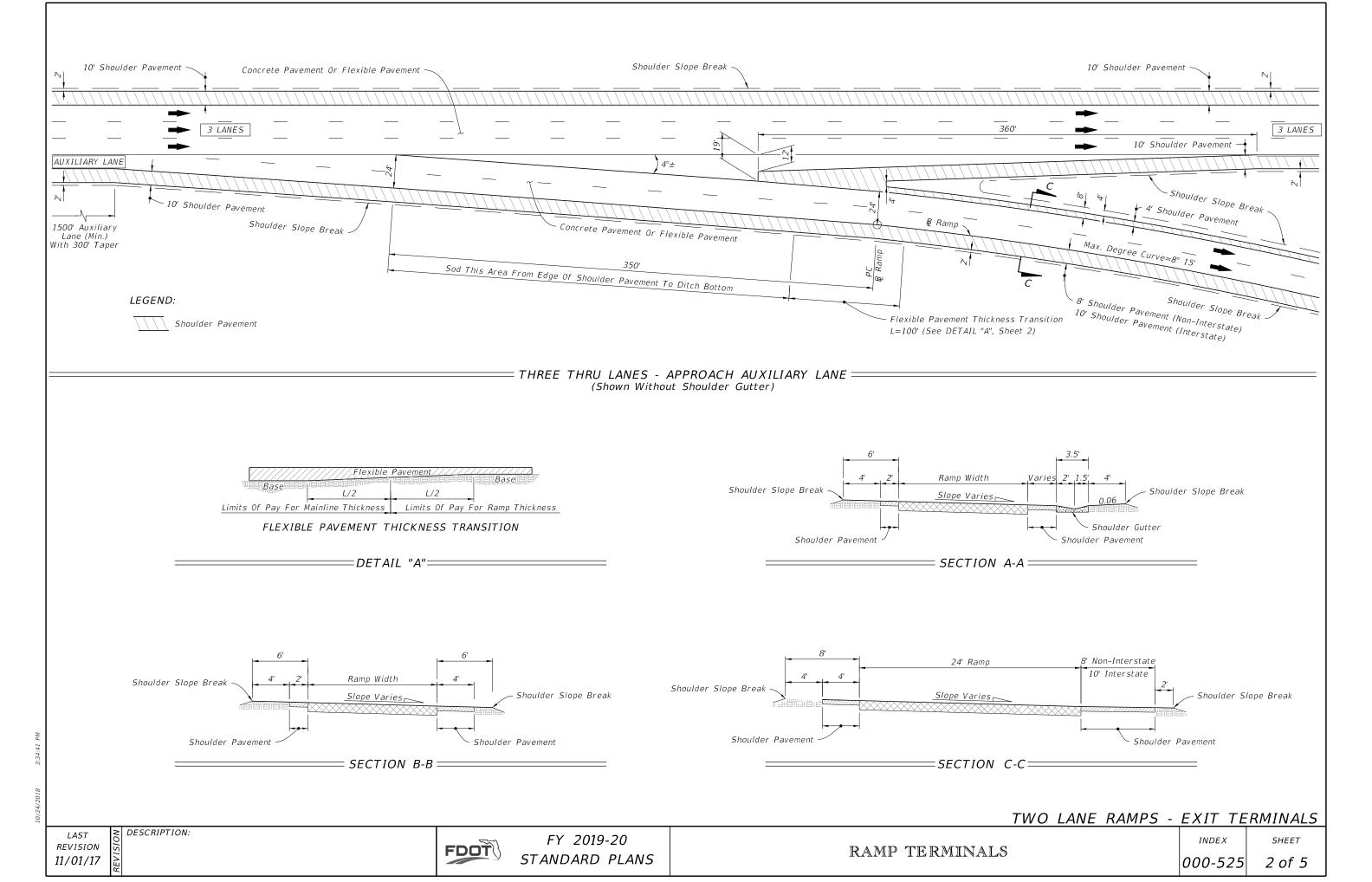
EXAMPLE SUPERELEVATION SECTIONS AND PROFILES FOR LOW SPEED HIGHWAYS

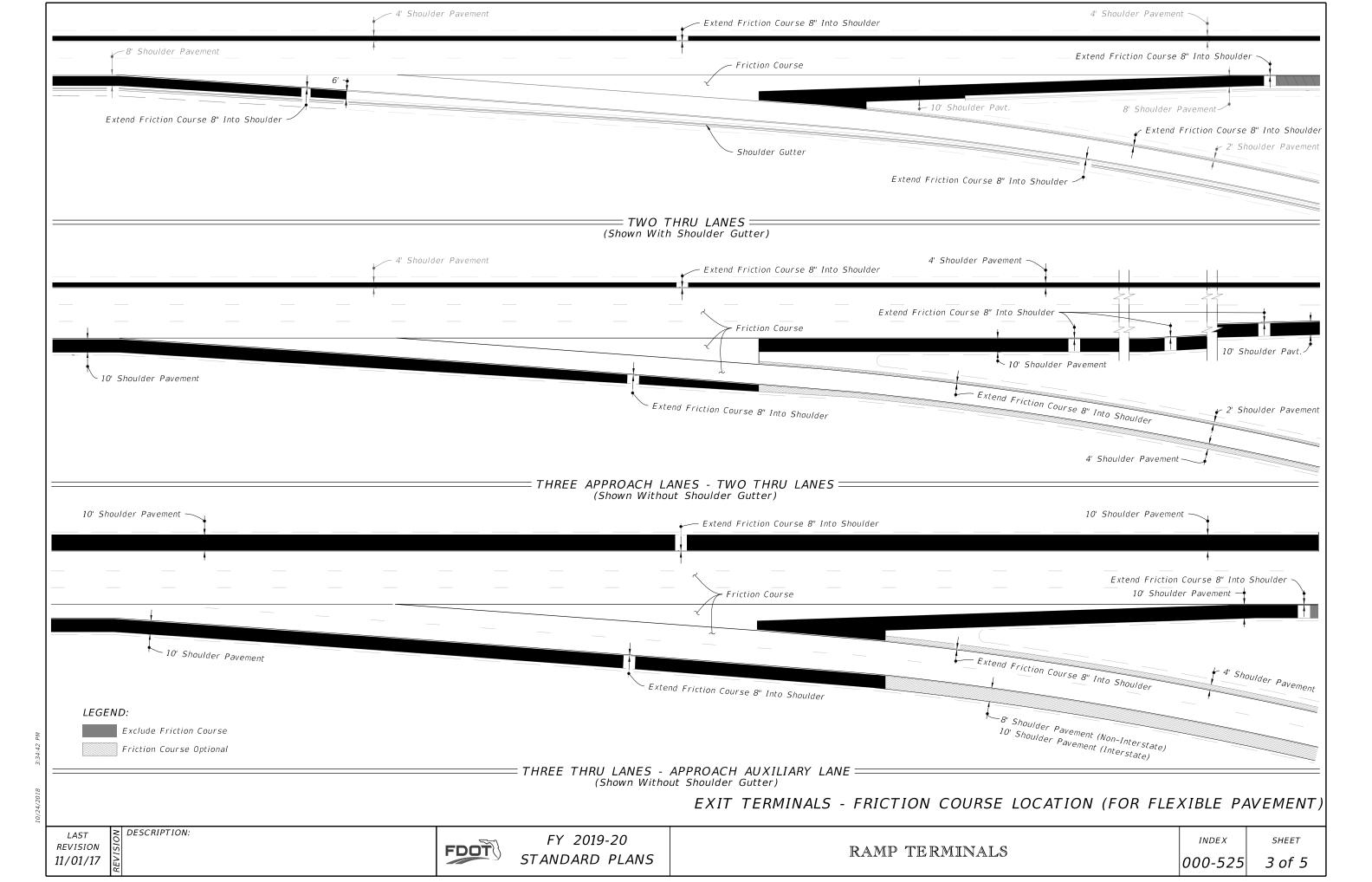
REVISION 11/01/18

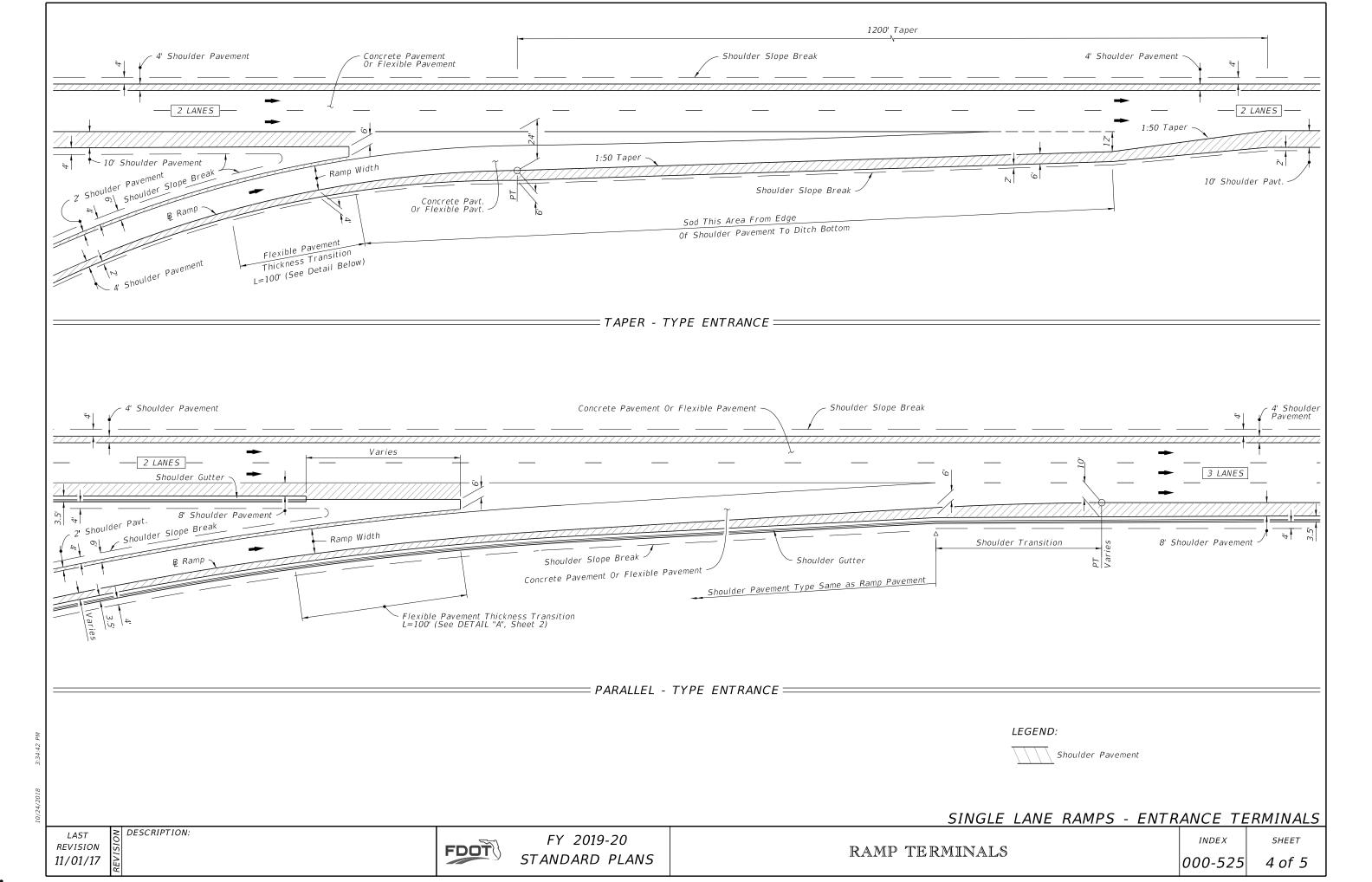
DESCRIPTION:

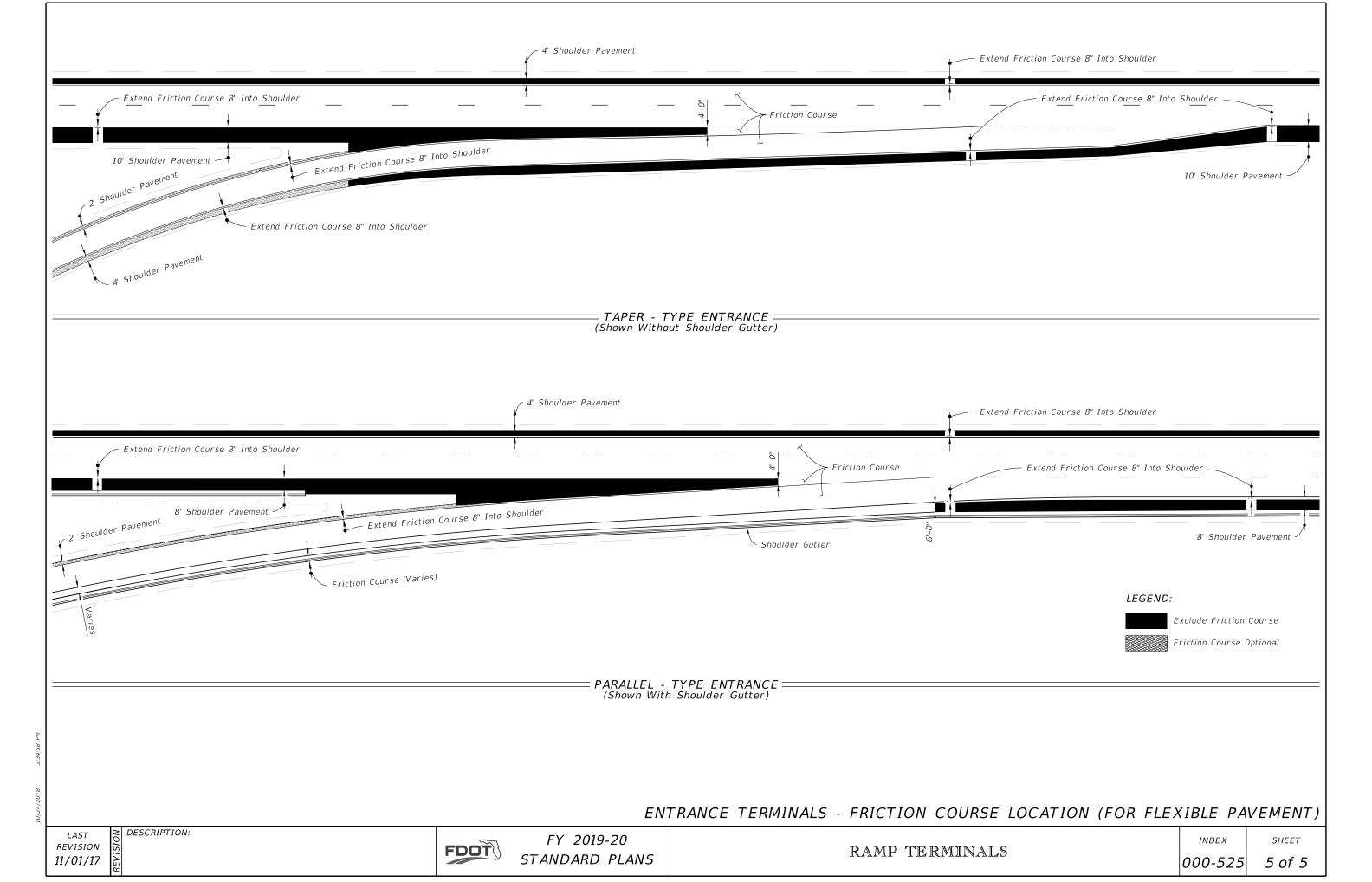
FDOT









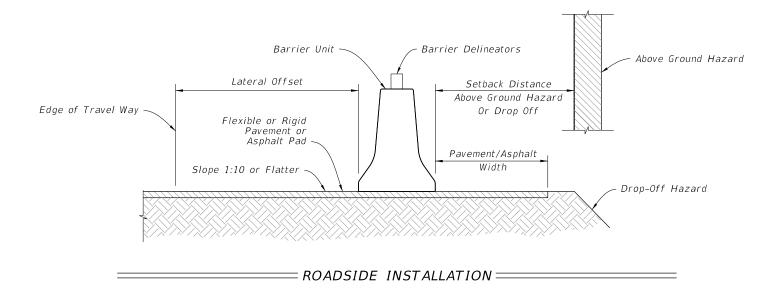


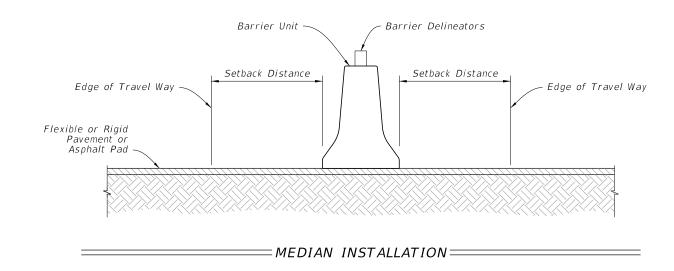
GENERAL NOTES:

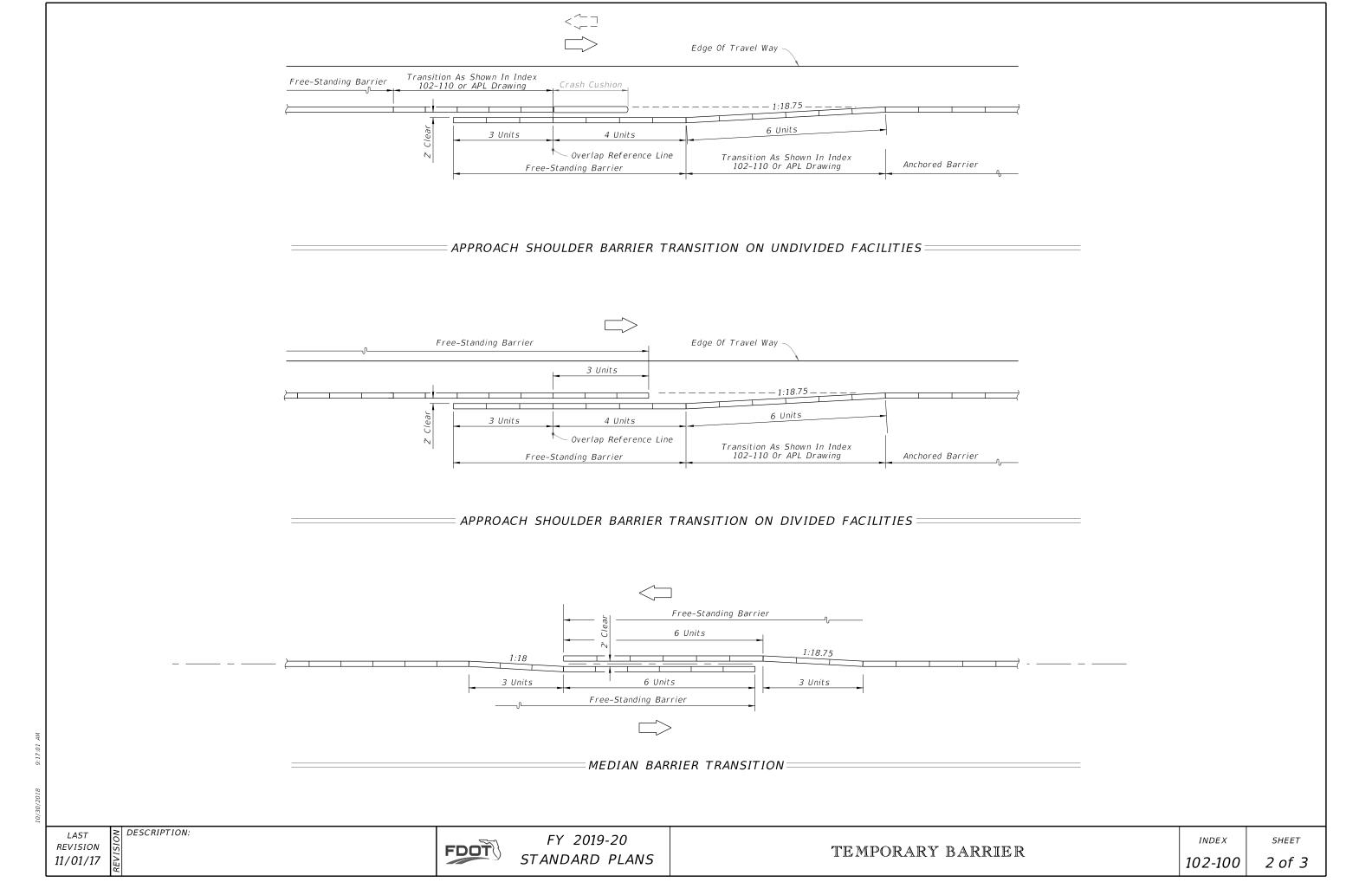
- 1. Temporary barrier systems may be any of the following:
- A. Type K Temporary Concrete Barrier System (Index 102-110) installed as either Free-Standing or Anchored.
- B. Proprietary Temporary Barrier Systems on the Approved Product List (APL).
- a. Concrete Barrier (Free-Standing or Anchored)
- b. Steel Barrier (Anchored)
- c. Water Filled Barrier (Free-Standing)
- 2. Where existing flexible pavement is not present, construct a minimum 2" thick temporary Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification 339 with the exception that the use of a pre-emergent herbicide is not required.
- 3. For Barrier Delineators, see Specification 102. Mount on top of temporary barriers. Color must match adjacent longitudinal pavement marking.
- 4. Remove all grass debris, loose dirt, and sand for the pavement, bridge deck, or asphalt pad surface within the barrier footprint just prior to placement of the temporary barrier.
- 6. Ensure the setback distance is clear of any grass, construction debris, stockpiled materials, equipment, and objects.
- 7. Transitions are required between Type K Barrier and free-standing, anchored, back-filled or other types of temporary barrier. See Index 102-110 for transitions between Type K Barrier and permanent bridge or traffic railing. Refer to the APL for transitions allowed for Proprietary Temporary Barrier Systems.
- 8. Anchoring (Bolting) of temporary barrier or crash cushions is not permitted on bridge superstructures that contain post-tensioned tendons within the concrete deck (top flange of concrete box girders) or on bridge superstructures consisting of longitudinally prestressed, transversely post-tensioned, solid or voided concrete slab units.
- 9. Anchor abutting segments of temporary barrier terminated with a Crash Cushion as shown in Index 102-110 or the APL.
- 10. The requirements of this Index do not apply to Temporary Low Profile Barrier, See Index 102-120.
- 11. Setback requirements below cover most Temporary Barrier options. Provide additional setback distance for APL products that require additional setback (deflection) space.

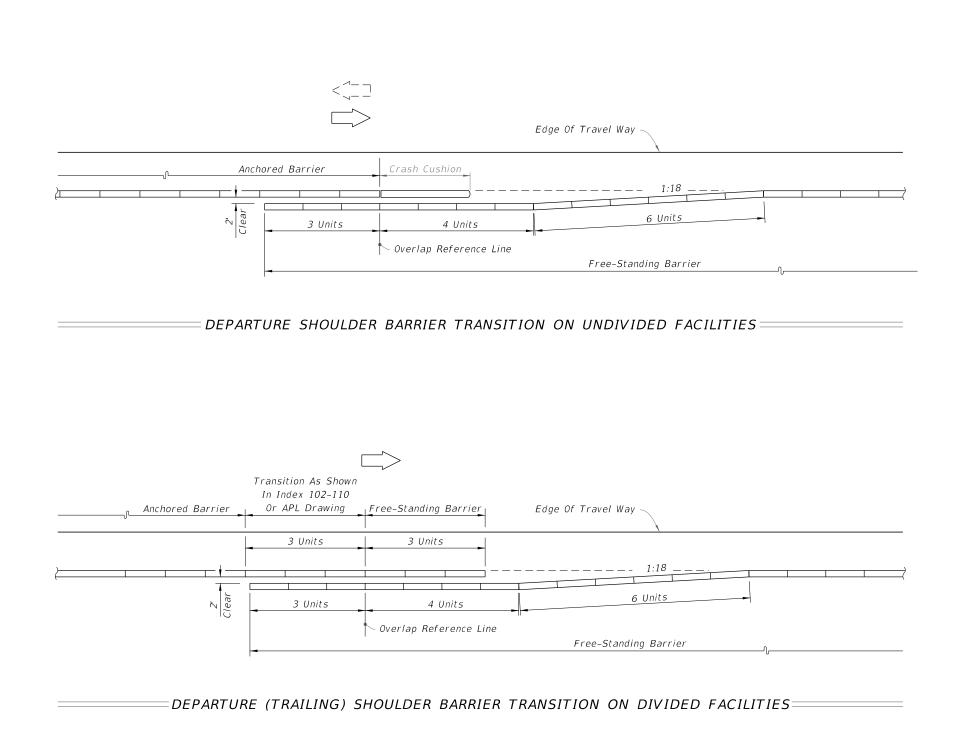
INSTALLATION DATA			
CONDITION	LATERAL OFFSET	SETBACK DISTANCE	PAVEMENT/ ASPHALT WIDTH
Anchored	2' Min.	2' Min.*	1' Min.
Free-standing	2' Min.	4' Min.	4' Min.

^{*} For Bridge Decks see Index 102-110 or APL.









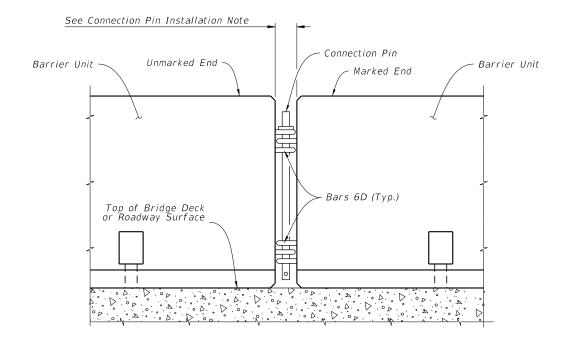
≥ DESCRIPTION: REVISION 11/01/17

GENERAL NOTES:

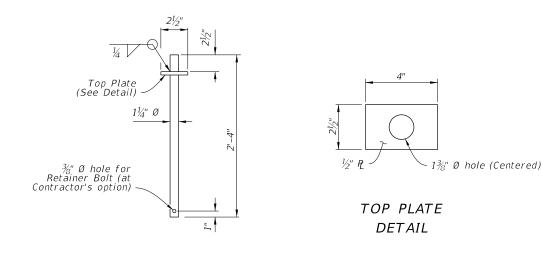
- 1. Meet the requirements of Index 102-100.
- 2. For fabrication details see Sheets 15 thru 17.
- 3. HANDLING: Do not lift or move the Barrier Units by using Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.
- 4. <u>CONNECTION PIN ASSEMBLY</u>: Use steel for Connection Pin and Top Plate assemblies in accordance with ASTM A36 or ASTM A709 Grade 36. Nondestructive testing of welds is not required. At the Contractor's option, a $\frac{3}{6}$ " diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.
- 5. <u>CONNECTION PIN INSTALLATION</u>: Initially set Barrier Units by using a 35%" wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 2). Do not use Barrier Units unconnected.
- 6. REUSE OF CONNECTION PINS AND STAKES: Connection pins and stakes may be reused if they have the structural integrity of new pins.
- 7. <u>REMOVAL OF BOLTS, STAKES AND KEEPER PINS:</u> Upon removal or relocation of Barrier Units, remove all Anchor Bolts and completely fill the remaining holes in bridge decks, approach slabs and roadway rigid pavements that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification 930 or with an Epoxy Resin Compound, Type F or Q, in accordance with Specification 926. If a flexible pavement is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.
- 8. Type K Anchored to Free-Standing transitions: Use the 3-3-2-1 Anchorage Transition Detail when transitioning Free-Standing and Anchored Units or when connecting Free-Standing runs to Crash Cushions, as shown in this Index.

NOTES FOR THRIE-BEAM GUARDRAIL SPLICE INSTALLATIONS:

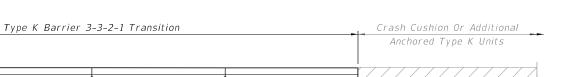
- 1. THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices meeting the requirements of specification 967 and as follows: Two panels per splice (One panel per side) of Class B (10 Gauge), or Four panels per splice (Two nested panels per side) of Class A (12 Gauge). Use a 12'-6" guardrail panel. Provide and install all other associated metallic guardrail components (Terminal Connectors, Shoulder Bolts, Hex Bolts and Nuts, Filler Plates, etc.) in accordance with Index 536-001. Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted.
- 2. <u>GUARDRAIL OFFSET BLOCKS</u>: Provide and install timber Offset Blocks meeting the requirements of Specification 967. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.
- 3. CONCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification 346, any Class, or a commercially available pre-bagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.



— DETAIL OF CONNECTION BETWEEN BARRIER UNITS



=CONNECTION PIN DETAIL====



1 Anchor 2 Anchors 3 Anchors 3 Anchors

= 3-3-2-1 ANCHORAGE TRANSITION DETAIL ====

730/2018 0:1.

LEGEND:

Dot indicates number and position of Bolts or Stakes

Free-Standing Barrier

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OVERLAY ADHESIVE BONDED ANCHOR INSTALLATION:

- Barrier Unit

11/4" Ø Adhesive-Bonded

Anchor with Heavy Hex Nut &

7" Min. **

Bridge Deck, Approach Slab or

Roadway Rigid Pavement

See PTFE Taping Detail

WITHOUT ASPHALT

OVERLAY

Embedment

3½" Sq. Top Plate Washer

NOTES FOR BOLTED INSTALLATIONS:

TYPICAL SECTION

Bridge deck shown, approach slab or rigid pavement similar; installation adjacent to drop-off shown, median transition installation similar.

LIMITATION OF USE: This installation technique can only be used on rigid pavement and concrete bridge decks as shown. Anchor Bolts must not be installed on both sides of the Barrier Units. Do not bolt down Barrier Units across bridge finger or modular expansion joints.

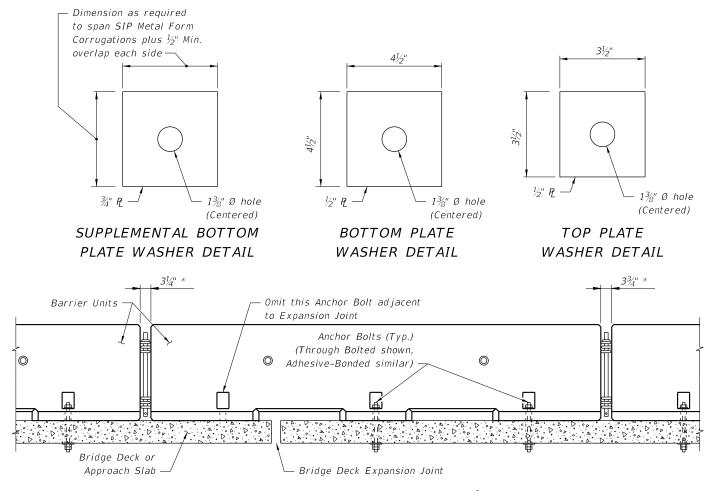
ANCHOR BOLTS, NUTS AND WASHERS: When using Adhesive-Bonded Anchor Bolts, use fully threaded rods in accordance with ASTM F 1554 Grade 36. Install Anchor Bolts for through bolting in accordance with ASTM A 307 or ASTM F 1554 Grade 36. Install nuts in accordance with ASTM A 563 or ASTM A 194. Install Flat Washers in accordance with ASTM F 436 and Plate Washers in accordance with ASTM A 36 or ASTM A 709 Grade 36

Install three (3) Anchor Bolts per Barrier Unit on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of Anchor Bolts required in Transition Installations see Sheets 8 and 9 and Index 102-100. Drilling through deck reinforcing steel to install Anchor Bolts is permitted. Unless otherwise shown in the Plans, at the Contractor's option Barrier Units may be installed by through bolting (where geometrically possible) or by the use of Adhesive-Bonded Anchor Bolts. Do not drill into or otherwise damage the tops of supporting beams or girders, bridge deck expansion joints or drains. Install Anchor Bolts and Nuts so that the maximum extension beyond the face of the Barrier Units is ½". Snug tighten the Nuts on the Anchor Bolts. For through bolted installations, snug tighten the double Nuts on the underside of the deck against each other to minimize the potential for loosening.

Omit one (1) Anchor Bolt within a single Barrier Unit if a conflict exists between the Anchor Bolt location and a bridge deck expansion joint or drain. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

Omit one (1) Anchor Bolt within a single Barrier Unit as shown in the Treatment at Bridge Deck Expansion Joint Schematic if the Barrier Unit straddles a bridge deck expansion joint. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

ADHESIVE-BONDING MATERIAL SYSTEMS: When using Adhesive Bonding Material Systems for Anchor Bolts, Use Type HSHV in accordance with Specification 937 and installed them in accordance with Specification 416. Prior to installation of the Barrier Units in the Plan location(s), install a demonstration Barrier Unit using the proposed production installation method, at a location approved by the Engineer. In lieu of the production test requirements of Specification 416, install six (6) Adhesive-Bonded Anchor Bolts in the demonstration Barrier Unit and test each Anchor Bolt with a 29,800 pound tensile proof load. Install and test additional demonstration Barrier Units when requested by the Engineer. Remove the demonstration Barrier Unit prior to testing the Anchor Bolts. Remove the test Anchor Bolts after testing as directed by the Engineer.



* To accommodate movement at Expansion Joint, set Barrier Units with 3¾" gap at locations shown.

TREATMENT AT BRIDGE DECK EXPANSION JOINT SCHEMATIC

ANCHORED INSTALLATIONS - BOLTED ===

REVISION 11/01/17

DESCRIPTION:

** Wrap threads with a single

overlapping laver of PTFE

tape to facilitate removal

Asphalt Overlay

7" Min **

Embedment

See PTFE

WITH ASPHALT

Taping Detail

of anchors.

Tape

PTFE

TAPING DETAIL

- Barrier Unit

Bridge

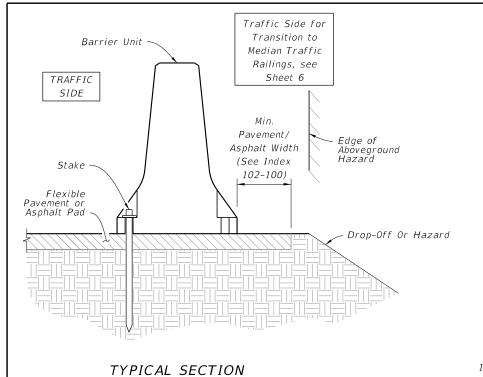
Deck

THROUGH BOLTED ANCHOR

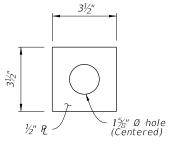
22

Barrie

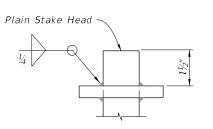
Unit



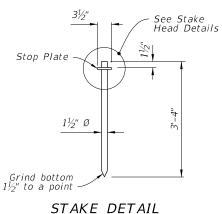
TYPICAL BRIDGE SECTION

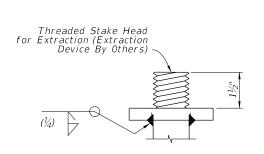


STOP PLATE DETAIL



PLAIN STAKE HEAD DETAIL





OPTIONAL EXTRACTION STAKE HEAD DETAIL

NOTES FOR STAKED INSTALLATIONS:

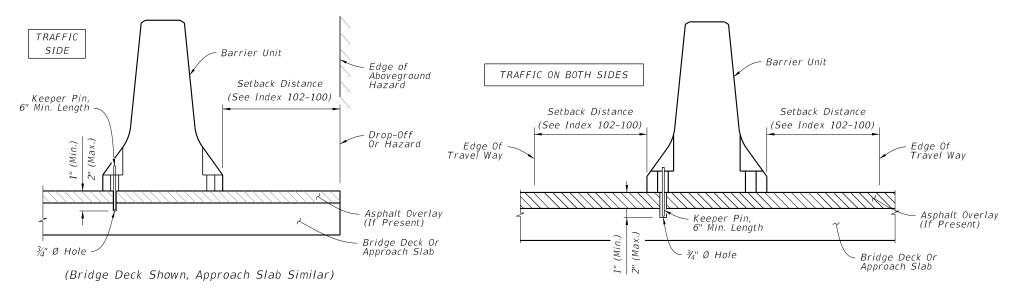
LIMITATION OF USE: This installation technique can only be used on flexible pavement or an Asphalt Pad as shown. Stakes must not be installed on both sides of the Barrier Units.

STAKES: Provide steel for Stake assemblies in accordance with ASTM A 36 or ASTM A 709 Grade 36. Weld in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Welding metal are E60XX or E70XX. Nondestructive testing of welds is not required.

Install three (3) Stakes on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of stakes required in Transition Installations see Sheets 4, 5 and 6 and Index 102-100. Install Stakes so that the Stop Plate is snug against the bottom of the Anchor Blockout.

BURIED UTILITIES: Prior to installation of Stakes verify locations of all adjacent buried utilities, drainage structures, pipes, etc. If conflicts between Stake locations and buried elements exist, a maximum of two (2) Stakes within a single Barrier Unit may be omitted if the adjacent Barrier Units are installed with the standard three (3) Stakes.

ANCHORED INSTALLATIONS - STAKED:

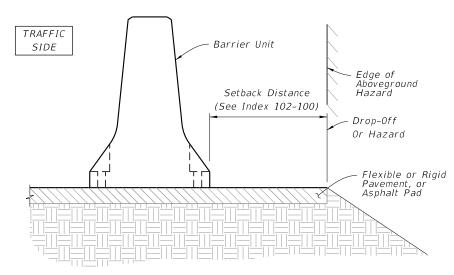




NOTES FOR FREE-STANDING INSTALLATION:

- 1. For Bridge Decks only, use Keeper Pins that are V_2 " diameter, smooth steel bar in accordance with ASTM A36 or ASTM A709 Grade 36. As directed by the Engineer in order to limit vibration induced translation of the Barrier Units, install one (1) Keeper Pin per Barrier Unit as shown.
- 2. If traffic is on both sides of the Barrier (i.e. Median Installation), alternate Keeper Pin locations from side to side of Barrier Units along the length of the installation. If traffic is on only one side of the barrier install keeper pins on the traffic side as shown.
- 3. Do not drill into or otherwise damage bridge deck expansion joints or drains.

FREE-STANDING INSTALLATION =



TYPICAL ROADWAY SECTION

REVISION 11/01/17

DESCRIPTION:

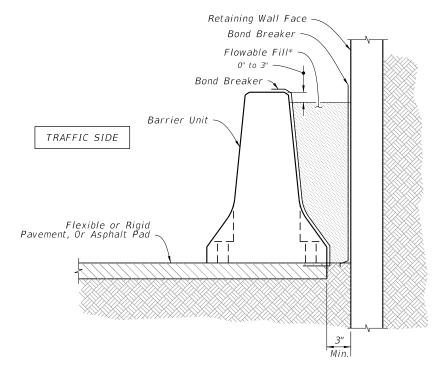
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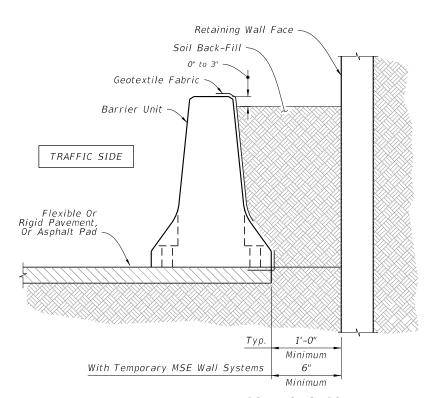
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TYPICAL SECTION ADJACENT TO RETAINING WALL WITH FLOWABLE FILL BACK-FILL

*FLOWABLE FILL: Provide Excavatable Flowable Fill in accordance with Specification 121.

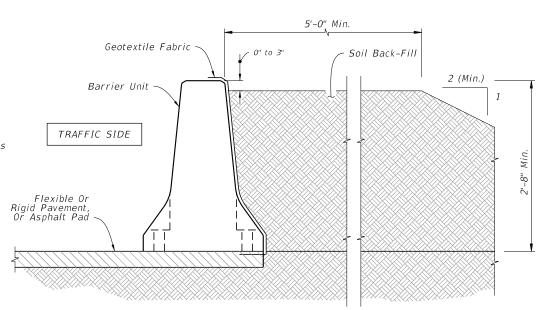
FLOWABLE FILL BACK-FILL ROADSIDE INSTALLATIONS



NOTES FOR SOIL BACK-FILLED ROADWAY INSTALLATIONS:

SOIL BACK-FILL MATERIAL: Provide Back-Fill Material consisting of any available clean soil. Compact Back-Fill Material until the soil mass is firm and unyielding. Provide erosion control as specified in the Plans. If none is specified in the Plans, provide erosion control as required to maintain the integrity of the Back Fill embankment.

GEOTEXTILE FABRIC: Provide Type D-5 Geotextile Fabric in accordance with Specification 985 to contain Back Fill Material behind Barrier Units. Geotextile Fabric may be continuous over the length and height of the installation or may be individual pieces as required to cover the Lift / Drain Slots and open vertical joints between Barrier Units.



TYPICAL SECTION WITH SOIL BACK-FILL

TYPICAL SECTION ADJACENT TO RETAINING WALL WITH SOIL BACK-FILL

SOIL BACK-FILLED ROADSIDE INSTALLATIONS

LAST **REVISION** 11/01/17

DESCRIPTION:

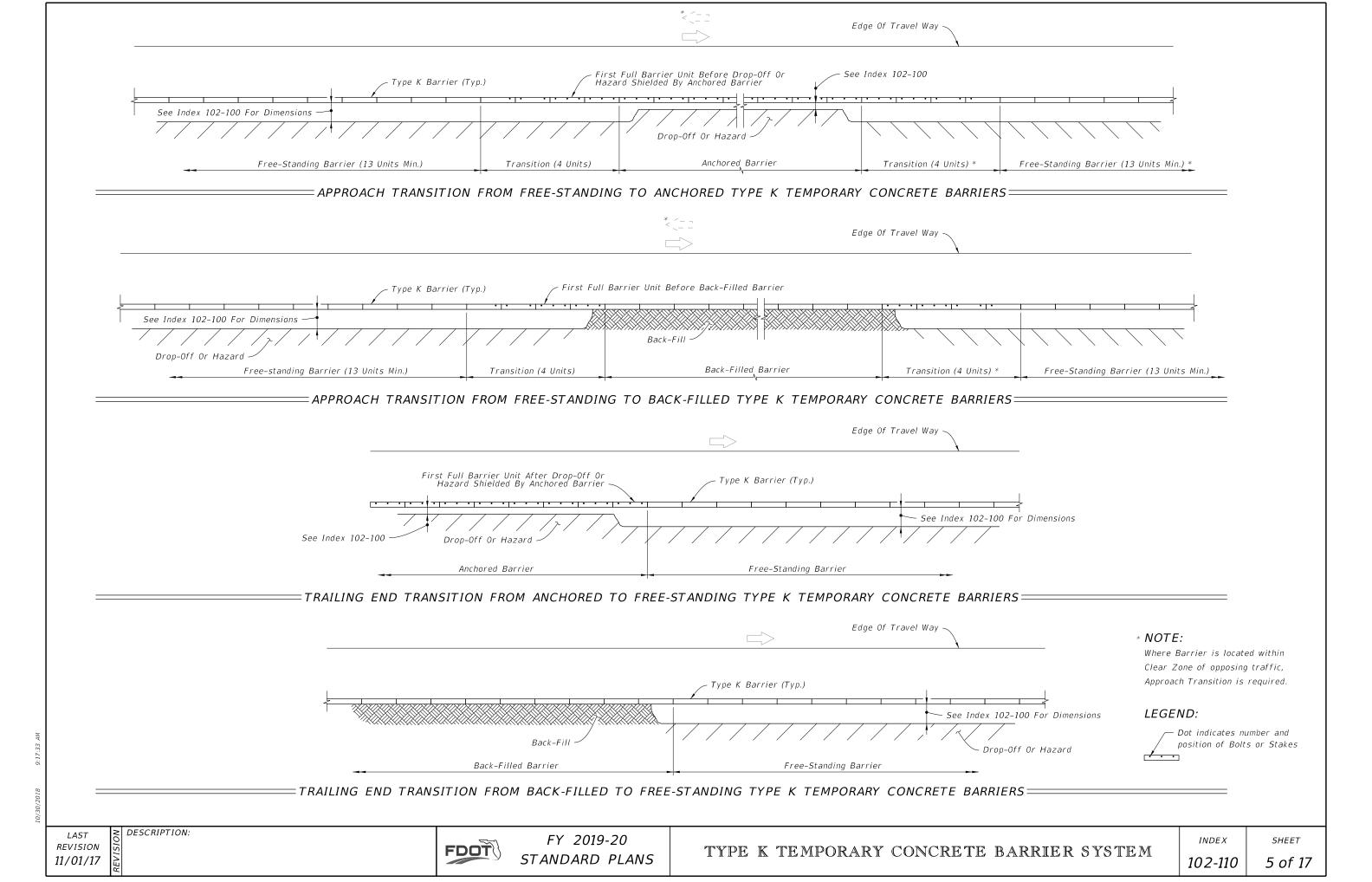
FY 2019-20 STANDARD PLANS

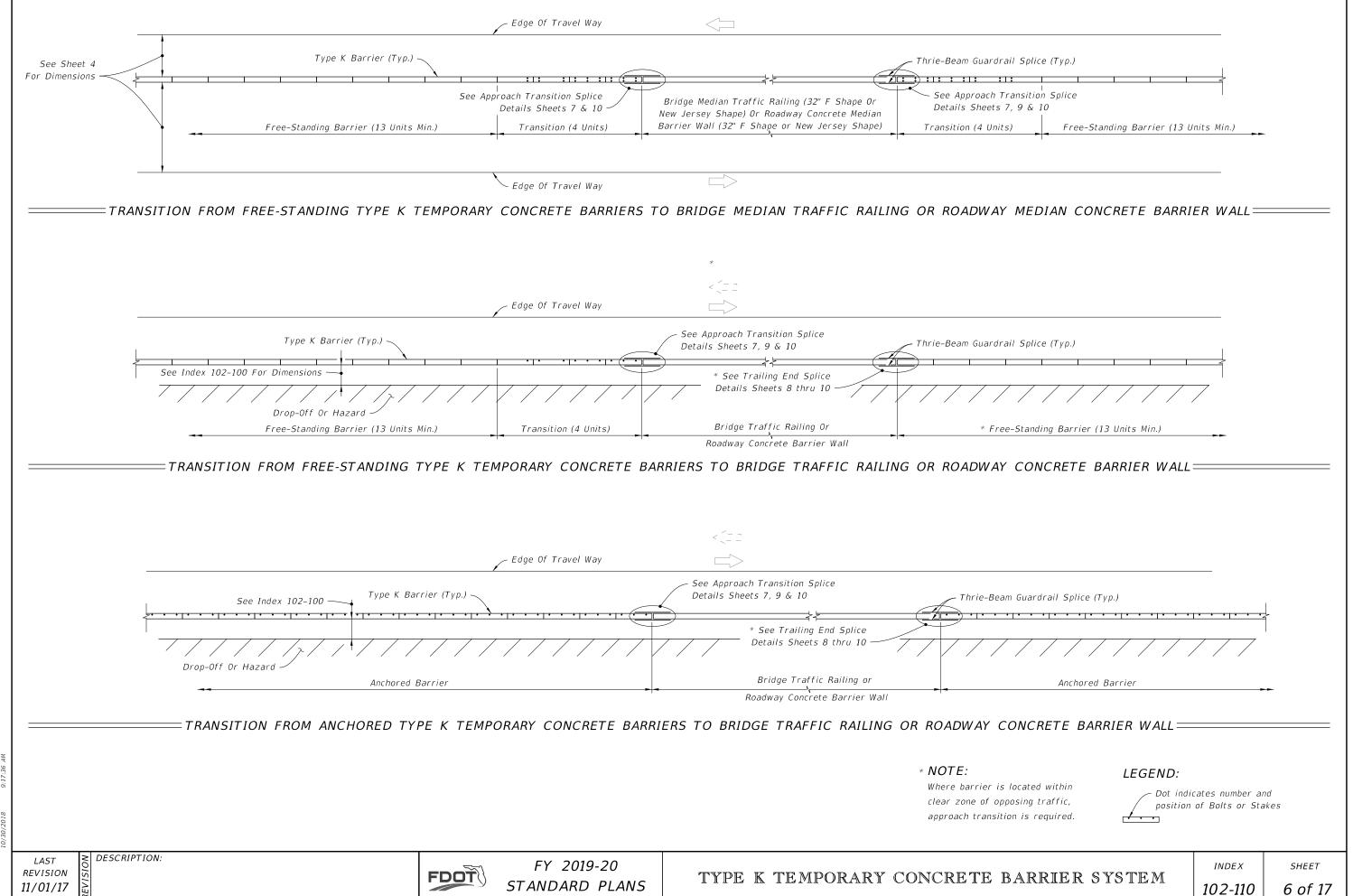
TYPE K TEMPORARY CONCRETE BARRIER SYSTEM

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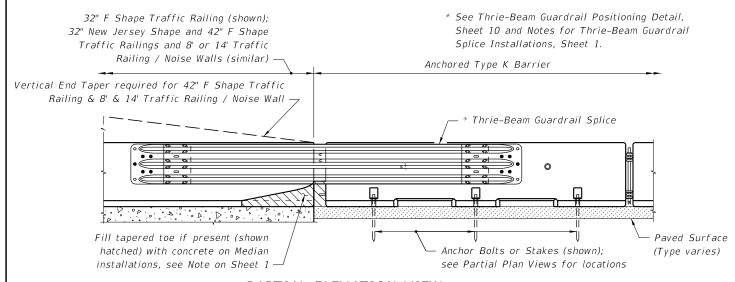




PARTIAL PLAN VIEW AT MEDIAN TRAFFIC RAILING

See Sheet 10 for Section A-A, 32" F Shape Traffic Railing (shown); Section B-B and Section C-C. 32" New Jersey Shape and 42" F Shape Traffic Railings and 8' or 14' Traffic Railing / Noise Walls (similar) -Anchored Type K Barrier * Thrie-Beam Guardrail Splice A | bolted to guardrail В — Offset Block or Stakes Align Top of Type K Barrier Unit with Traffic Railing at its end

PARTIAL PLAN VIEW AT SHOULDER TRAFFIC RAILING



PARTIAL ELEVATION VIEW

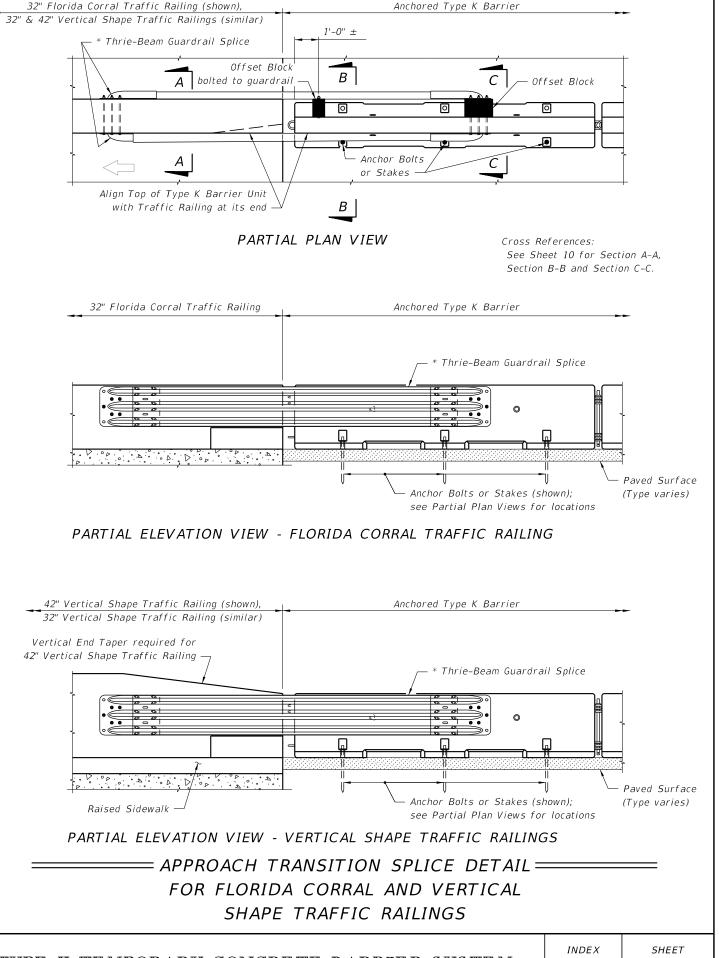
= APPROACH TRANSITION SPLICE DETAIL ==FOR F AND NEW JERSEY SHAPE TRAFFIC RAILINGS AND 8' & 14' TRAFFIC RAILING / NOISE WALLS (CONCRETE BARRIER WALL SIMILAR)

≥ DESCRIPTION: **REVISION** 11/01/17

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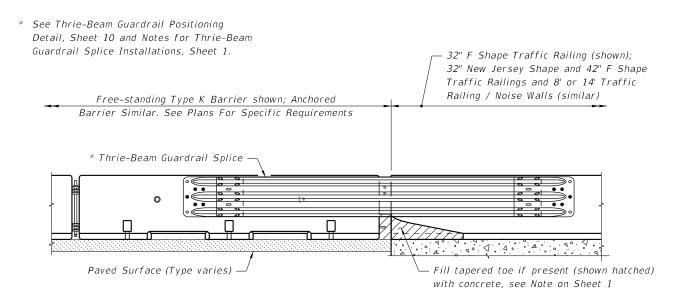
FY 2019-20 STANDARD PLANS

Cross References:



Anchored Type K Barrier

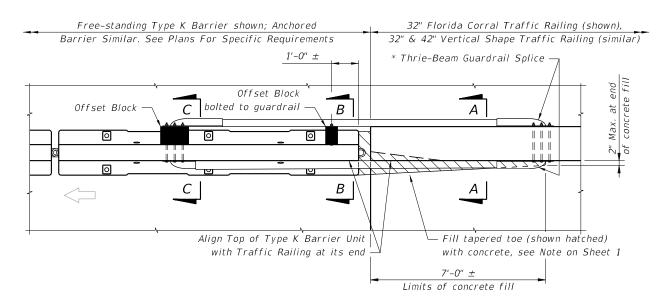
PARTIAL PLAN VIEW



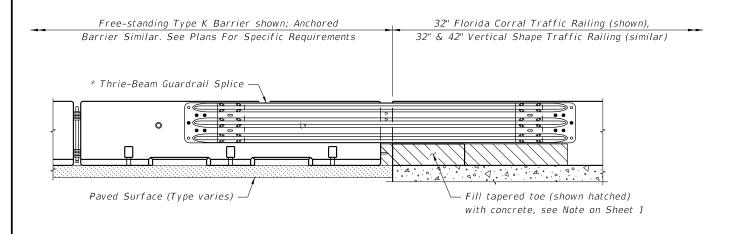
PARTIAL ELEVATION VIEW

Cross References: See Sheet 10 for Section A-A, Section B-B and Section C-C.

— TRAILING END SPLICE DETAIL —— FOR F AND NEW JERSEY SHAPE TRAFFIC RAILINGS AND 8' & 14' TRAFFIC RAILING / NOISE WALLS



PARTIAL PLAN VIEW



PARTIAL ELEVATION VIEW

Cross References: See Sheet 10 for Section A-A, Section B-B and Section C-C.

= TRAILING END SPLICE DETAIL =FOR FLORIDA CORRAL AND VERTICAL SHAPE TRAFFIC RAILINGS

REVISION 11/01/17

FDOT

FY 2019-20 STANDARD PLANS

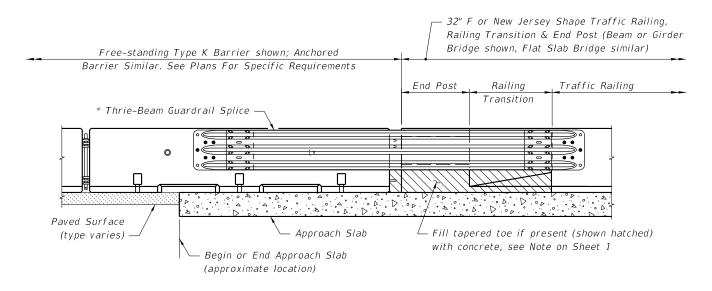
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PARTIAL PLAN VIEW

* See Thrie-Beam Guardrail Positioning Detail, Sheet 10 and Notes for Thrie-Beam Guardrail Splice Installations, Sheet 1.



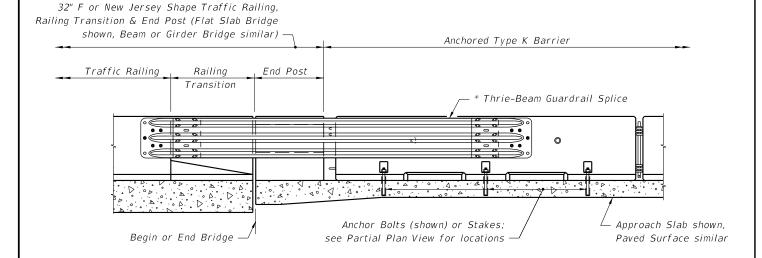
PARTIAL ELEVATION VIEW

Cross References: See Sheet 10 for Section B-B, Section C-C and Section D-D.

—TRAILING END SPLICE DETAIL—— FOR 32" F AND NEW JERSEY SHAPE TRAFFIC RAILINGS WITH RAILING TRANSITION AND END POST

32" F or New Jersey Shape Traffic Railing, Railing Transition & End Post (Flat Slab Bridge shown, Beam or Girder Bridge similar) Anchored Type K Barrier - * Thrie-Beam Guardrail Splice — Offset Block bolted to Guardrail Offset Block - Offset Block В Anchor Bolts or Stakes Align Top of Type K Barrier Unit with Traffic Railing at its end

PARTIAL PLAN VIEW



PARTIAL ELEVATION VIEW

Cross References: See Sheet 10 for Section B-B, Section C-C and Section E-E.

=APPROACH TRANSITION SPLICE DETAIL=== FOR 32" F AND NEW JERSEY SHAPE TRAFFIC RAILINGS WITH RAILING TRANSITION AND END POST

REVISION 11/01/17

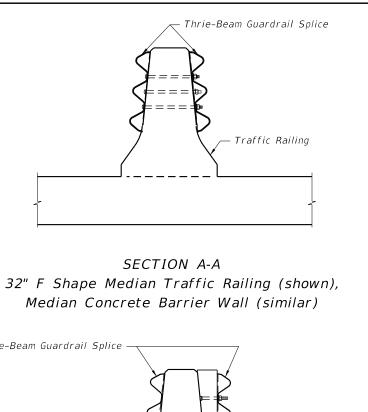
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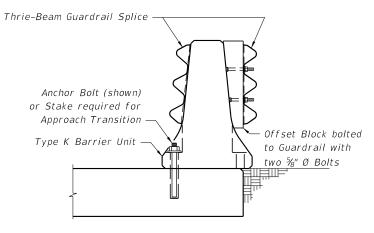
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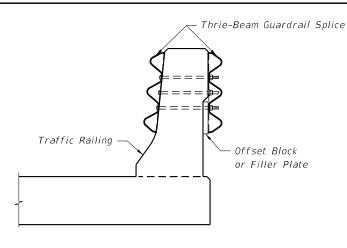
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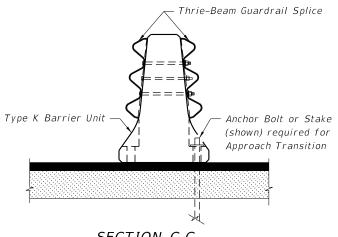




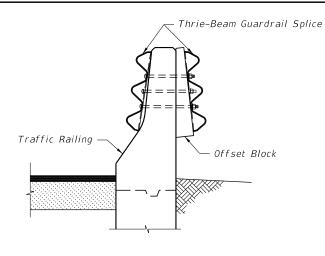
SECTION B-B Adjacent to Shoulder Traffic Railings



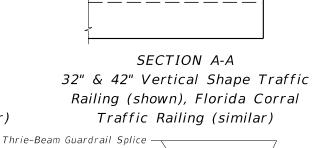
SECTION A-A 32" F Shape Traffic Railing (shown), 42" Traffic Railing and 8' & 14' Traffic Railing / Noise Walls (similar)



SECTION C-C Adjacent to 32" F or New Jersey Shape Median Traffic Railing or Median Concrete Barrier Wall



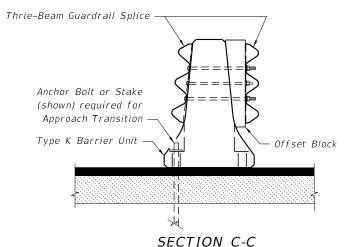
SECTION A-A 32" New Jersey Shape Concrete Barrier Wall (shown), 32" New Jersey Shape Traffic Railing & other Narrow Traffic Railings (similar)



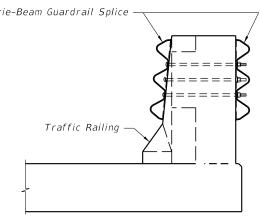
Thrie-Beam Guardrail Splice

Fill tapered toe (shown hatched) with concrete

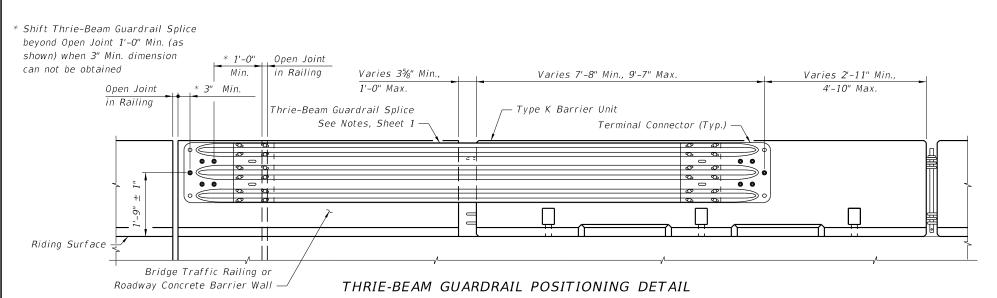
on Trailing Ends only

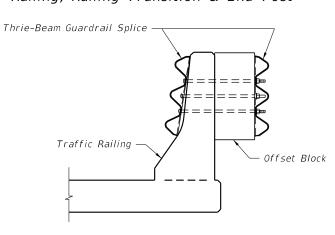


Adjacent to Shoulder Traffic Railings



SECTION D-D 32" F or New Jersey Shape Traffic Railing, Railing Transition & End Post





SECTION E-E 32" New Jersey Shape Traffic Railing (shown), 32" F Shape Traffic Railing (similar)

REVISION 11/01/17

DESCRIPTION:

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TYPE K TEMPORARY CONCRETE BARRIER SYSTEM

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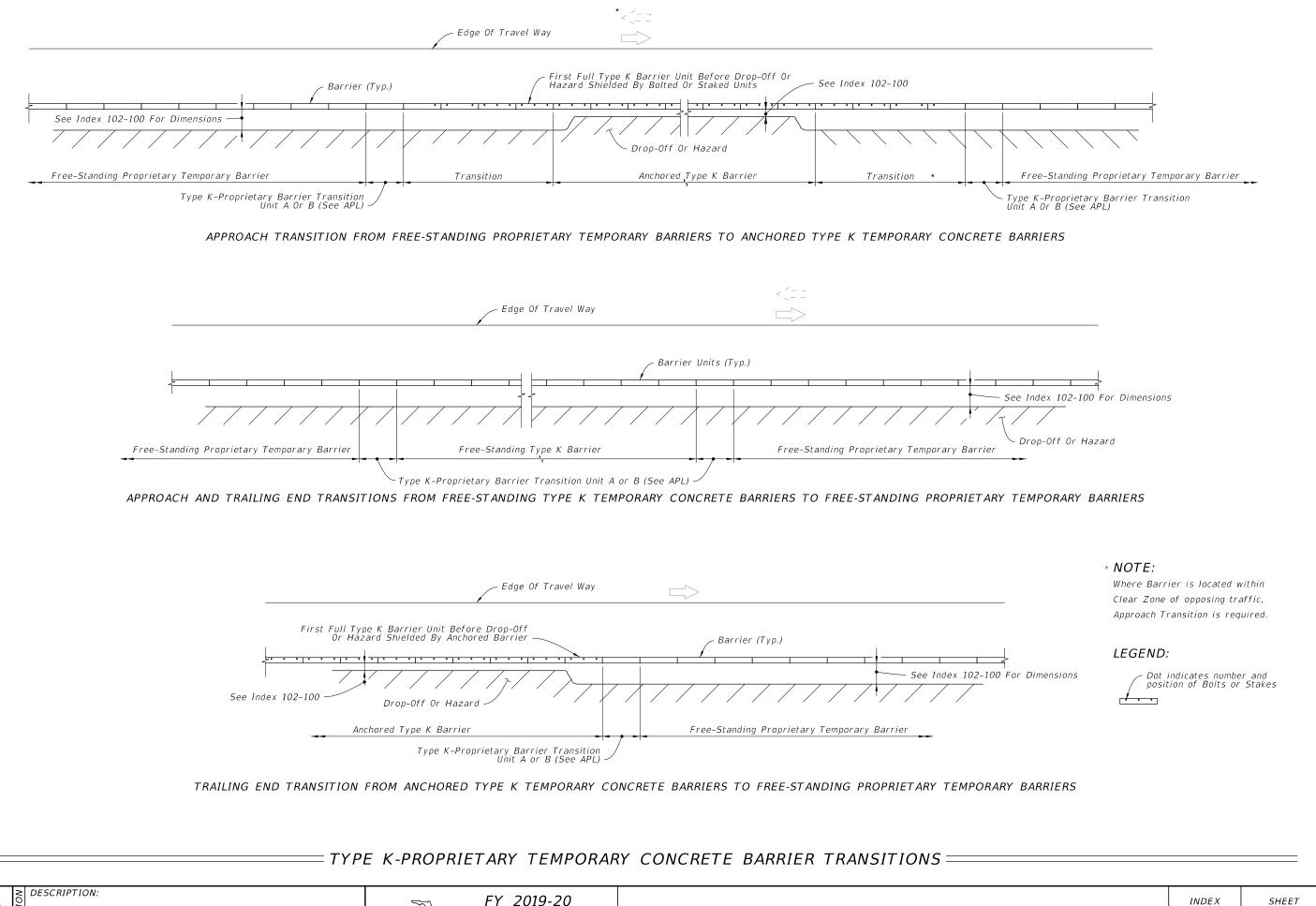
SHEET

Offset Block

Railing

or Filler Plate

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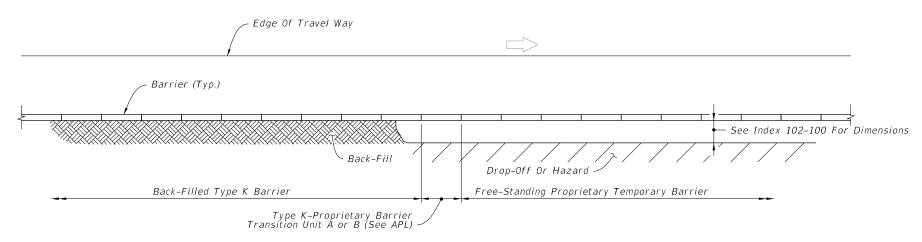


LAST **REVISION** 11/01/17

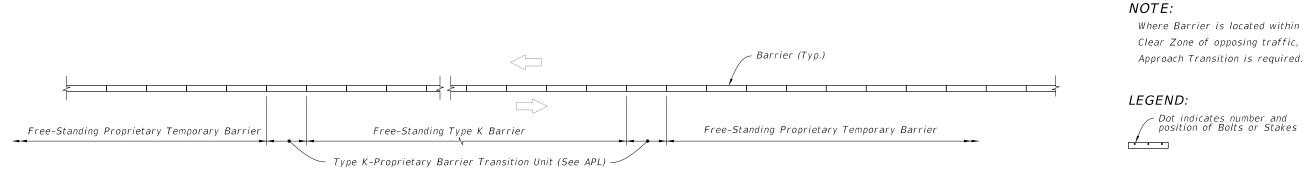
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APPROACH TRANSITION FROM FREE-STANDING PROPRIETARY TEMPORARY BARRIERS TO BACK-FILLED TYPE K TEMPORARY CONCRETE BARRIERS



TRAILING END TRANSITION FROM BACK-FILLED TYPE K TEMPORARY CONCRETE BARRIERS TO FREE-STANDING PROPRIETARY TEMPORARY BARRIERS



MEDIAN APPROACH AND TRAILING END TRANSITIONS FROM FREE-STANDING TYPE K TEMPORARY CONCRETE BARRIERS TO FREE-STANDING PROPRIETARY TEMPORARY BARRIERS



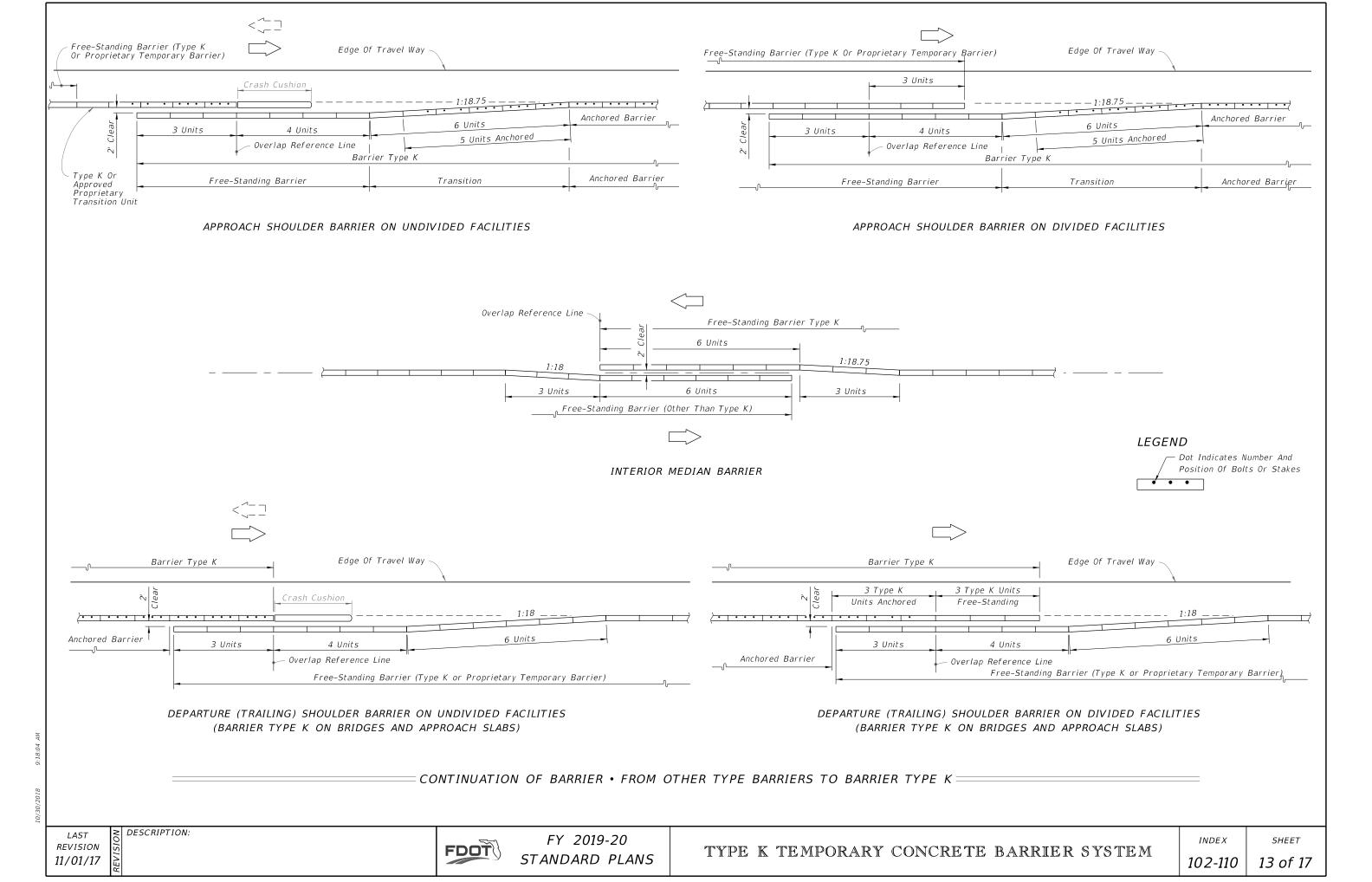
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TYPE & TEMPORARY CONCRETE BARRIER SYSTEM

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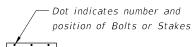
SHEET

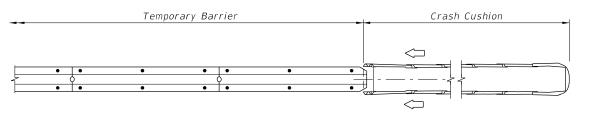




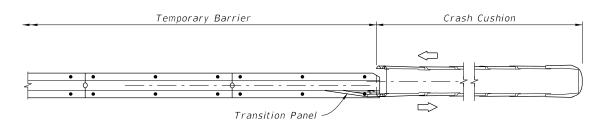
When subjected to reverse direction hits, construct Transition Panels from Temporary Barrier to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.

LEGEND:

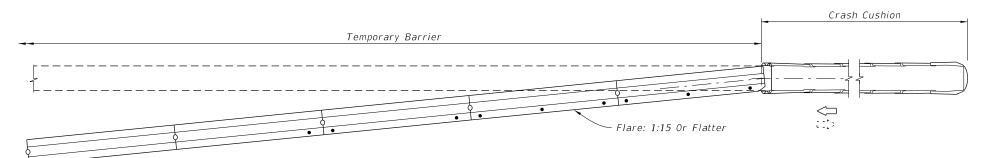




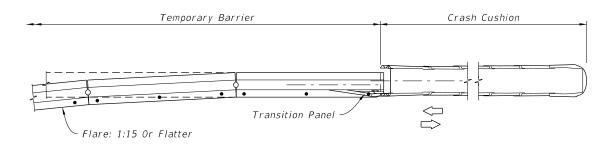
UNIDIRECTIONAL - SEPARATED TRAFFIC



BIDIRECTIONAL - SEPARATED TRAFFIC



TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED
OUTSIDE OPPOSING LANE CLEAR ZONE OR ONE-WAY TRAFFIC



TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED WITHIN OPPOSING LANE CLEAR ZONE

END TREATMENT WHEN SHIELDED BY A CRASH CUSHION

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)

SHIELDING ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION)

R

DESCRIPTION:

FABRICATION NOTES:

In order to maintain crashworthiness of the Barrier System, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

FABRICATOR PREQUALIFICATIONS:

- A. The Concrete Plant that meets the requirements;
- a. Specifications 450 for prestressed concrete
- b. Specification 105 for precast.

CONCRETE:

- A. Construct Barrier Units with Class IV concrete in accordance with Specification 346.
- B. Specification 346-10.2 through 346-10.4 are not applicable.
- C. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.

REINFORCING STEEL:

- A. Use only steel reinforcing that meet ASTM A 615, Grade 60, with the exception of Bars 6D1, 6D2 and 6D3.
- B. Bars 6D1, 6D2 and 6D3 use steel reinforcing that meets ASTM A 706, with the exception that a $2\frac{3}{4}$ " diameter pin must be used for the 180 degree bend test.
- C. After steel reinforcing fabrication, hot dip galvanized in accordance with Specification 962 or coated with a cold galvanizing compound in accordance with Specification 562, all or part of Bars 6D.
- D. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated.
- E. The minimum limit of galvanizing or coating is shown in the Bending Diagrams.
- F. Install Bars 6D within $\frac{1}{8}$ " of the plan dimensions.
- G. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.
- H. At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with Specification Section 931 and the details shown on Sheet 15 may be utilized in lieu of Bars 4A and 5B.
- I. All dimensions in the Bending Diagrams are out to out.
- J. Install all reinforcing steel with a 2" minimum cover, except as noted.

<u>LIFTING SLEEVE ASSEMBLY:</u>

- A. Inclusion of the Lifting Sleeve Assemblies is optional.
- B. Use steel in accordance with ASTM A 53 for the Pipe Sleeve.
- C. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.

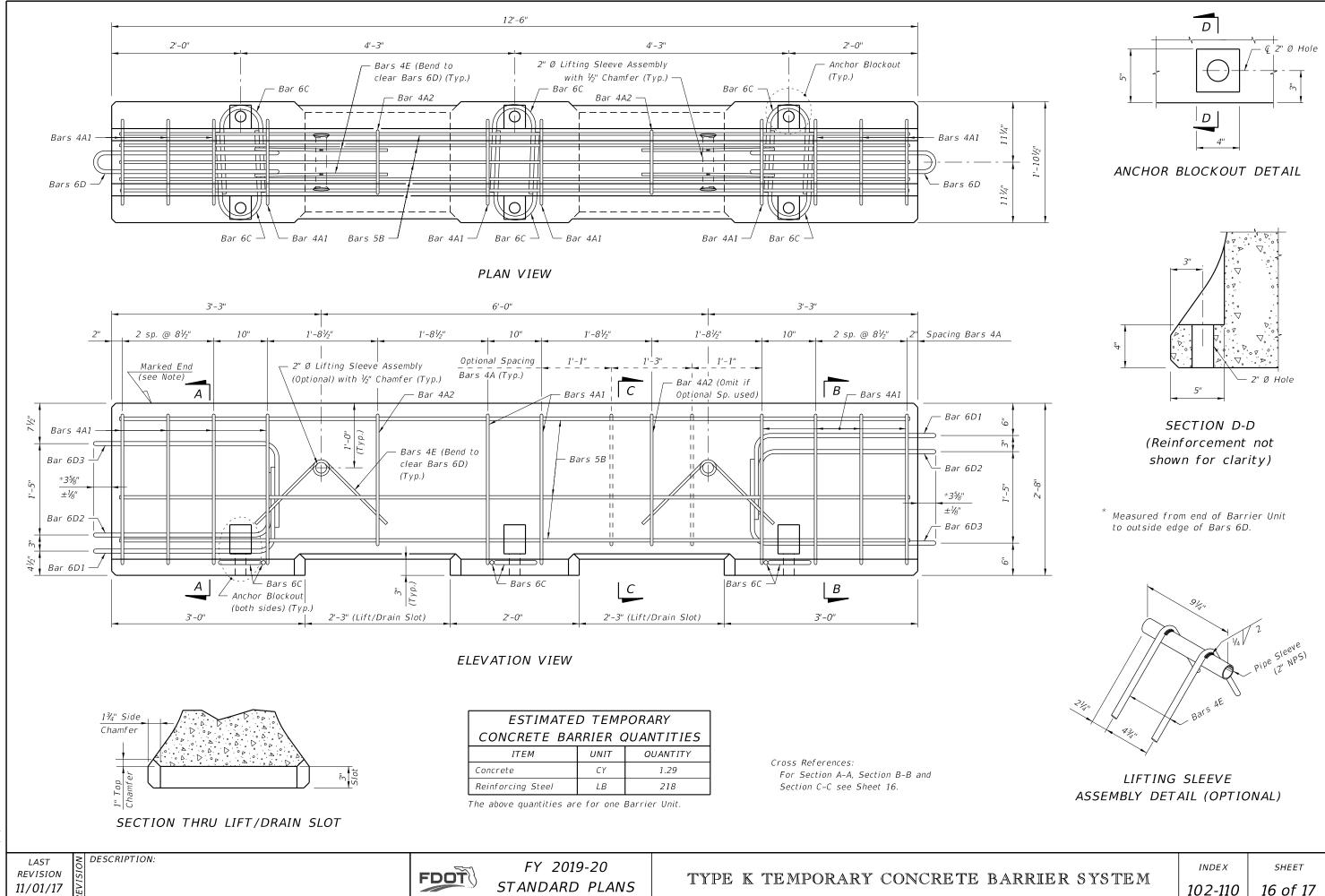
SURFACE FINISH:

- A. Construct Barrier Units in accordance with Specification Sections 400 and 521.
- B. Finish the top and sides of the Barrier Units with a General Surface Finish.
- C. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish.
- D. Use stationary metal forms or stationary timber forms with a form liner.

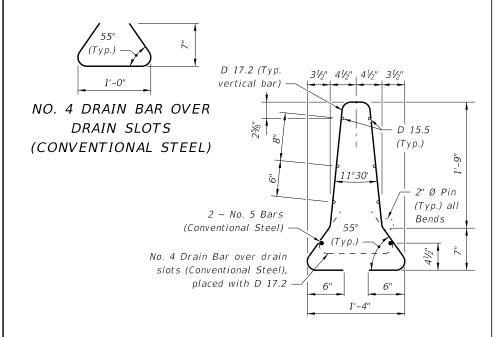
MARKING:

- A. Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall.
- B. Ink stamps are not allowed.
- C. Permanently mark with the following information:
- Tvpe K1
- Fabricator's name or symbol
- Date of manufacture (day, month and year)

DESCRIPTION:



ALTERNATE REINFORCING STEEL DETAIL WELDED WIRE REINFORCEMENT

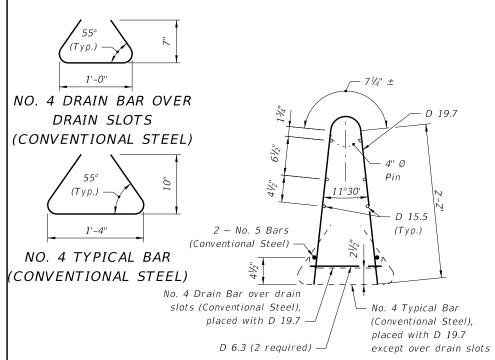


NOTES:

Place 2 ~ No. 5 Bars (12'-3" long) in bottom of Welded Wire Reinforcement cage as shown.

Match D17.2 spacing to Bars 4A in the Elevation View, Sheet 15. Field trim D17.2 to clear drain slot by 2".

CONFIGURATION ONE



NOTES:

Place 2 ~ No. 5 Bars (12'-3" long) tied to D 19.7 inside of bottom Welded Wire Reinforcement cage as shown.

Match D19.7 spacing to Bars 4A in the Elevation View, Sheet 15.

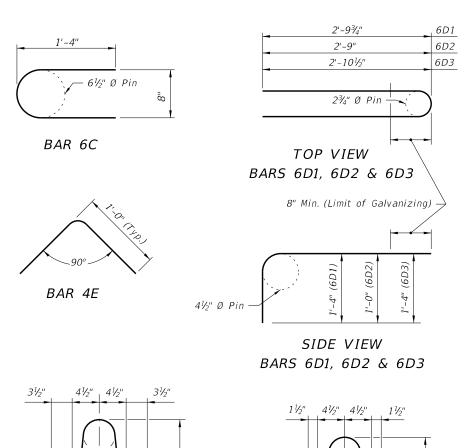
Field trim D19.7 to clear drain slot by 2".

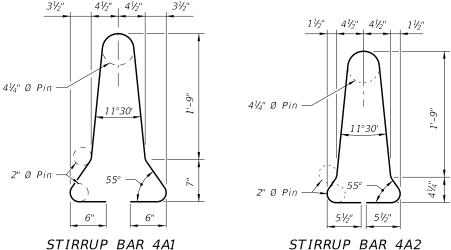
DESCRIPTION:

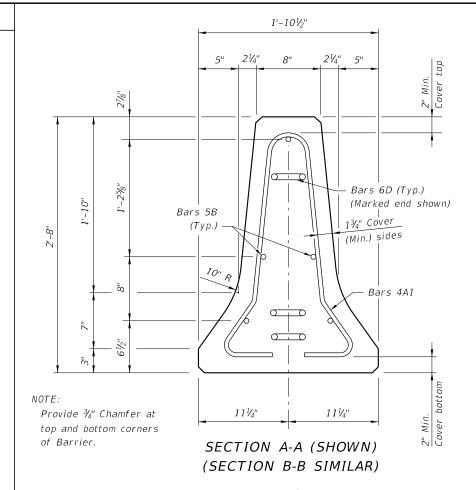
CONFIGURATION TWO

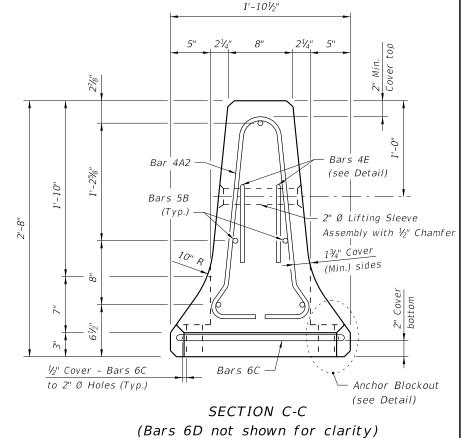
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL					
MARK	SIZE	NUMBER	LENGTH		
A1	4	10	6'-1"		
A2	4	2	5'-5"		
В	5	5	12'-3" (Straight)		
С	6	6	3'-1"		
D1	6	2	8'-4"		
D2	6	2	7'-6"		
D3	6	2	8'-6"		
E	4	4	2'-0"		









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GENERAL NOTES

- 1. Pursuant to 35 United States Code, Chapter 18, also known as the Bayh Dole Act of 1980, the non mountable curb was developed through federal funding. The 'Portable Temporary Low Profile Barrier For Roadside Safety' is a licensed design by the University Of Florida. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This Index is provided by the Florida Department Of Transportation solely for use by the Department and its assignees. The purpose for this Index is to indicate the approval of use of the barrier on the State Highway System; to provide sufficient pictorials for identifying the barrier unit; and, to provide general installation geometry for the barrier.
- 3. This legally mandated relationship is unique to federally funded University patents that Department contractors use on Contracts. Pursuant to federal law, the University may pursue royalties for a valid patent. Only those barrier units cast by producers licensed by the University Of Florida will be allowed for installation on the State Highway System in Florida. Barrier wall units shall conform to Section 521 of the Standard Specification and shall be produced in Department-approved plants with quality control plans for precasting concrete barrier walls. Each barrier wall unit shall be permanently marked with an identification that is traceable to the manufacturer, the producing precast concrete plant and the date of production. This permanent identification mark will serve as certification that the unit has been manufactured in accordance with University of Florida drawings and specifications, and the approved quality control program.
- 4. The low profile barrier is to be installed only with hardware and accessories furnished by the licensed barrier producer. Units shall be used for no purpose other than as interconnected segments in a run of barrier. Low profile barrier wall units shall maintain firm contact with adjoining units. Nuts on tensioning rods shall be installed snug tight.
- 5. The low profile barrier is applicable for work zone speeds of 45 mph or less.
- 6. If the plans specify Low Profile Barrier then substitution with other barrier types is not
- 7. Tubular markers shall be orange in color and installed along the run of barrier at the ends and at 50' centers on tangents and 25' centers on radii. The markers shall be fixed to the top of the barrier by an adhesive or other method approved by the engineer. Approach end units shall be marked with a Type I object marker. The cost of the tubular markers and Type I object marker shall be included in the cost of the low profile barrier.
- 8. Information regarding licensing, shop drawings, specifications, quality control and certification of compliance can be obtained from the University Of Florida: Office of Technology Licensing, P.O. Box 115500, Gainesville, Florida, 32611–5500. Telephone: 352-392-8929, Fax: 352-392-6600. Reference UF#11052.
- 9. The Portable Temporary Low Profile Barrier For Roadside Safety shall be paid for under the contract unit price for Barrier Wall (Temporary) Low Profile Concrete, LF, and will be full compensation for furnishing, installing, maintaining and removing barrier wall.
- 10. Deflection space shall be kept clear of any grass, construction debris, stockpiled materials, equipment, and objects.



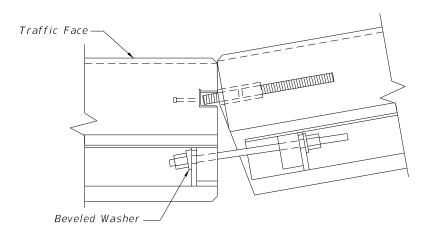
BACKSIDE AND END PICTORIAL VIEWS

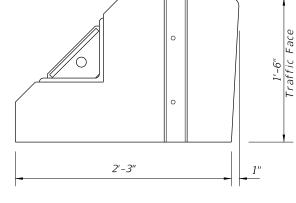
PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

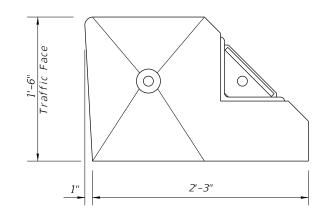
Unit Length 12.00

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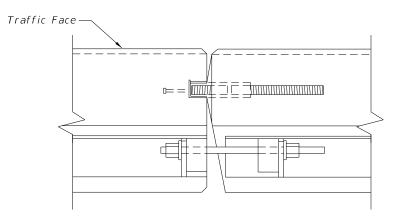




CONCAVE CONNECTION

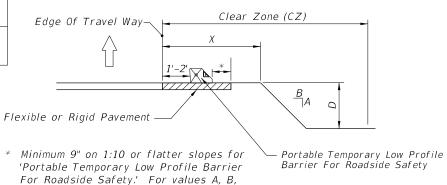
FLAT FACE FEMALE END

BEVELED FACE MALE END





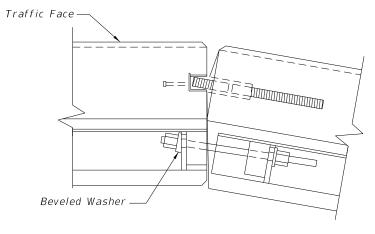
END VIEWS



LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement.

ASPHALT PAD: Where existing pavement is not present, construct 2" Asphalt Pad using miscellaneous asphalt pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. Payment for asphalt pad will be included in the cost of the barrier.

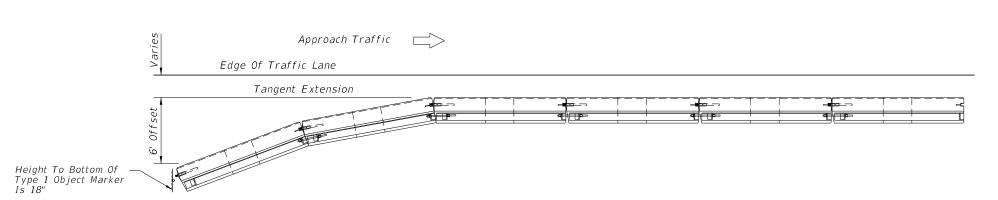
PARALLEL CONNECTION



CONVEX CONNECTION

PLAN VIEWS OF CONNECTIONS

DEFLECTION SPACE AT DROP-OFFS



PLAN VIEW OF APPROACH END OFFSET

D and X see Index 102-600.

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

LAST **REVISION** 11/01/17

DESCRIPTION:

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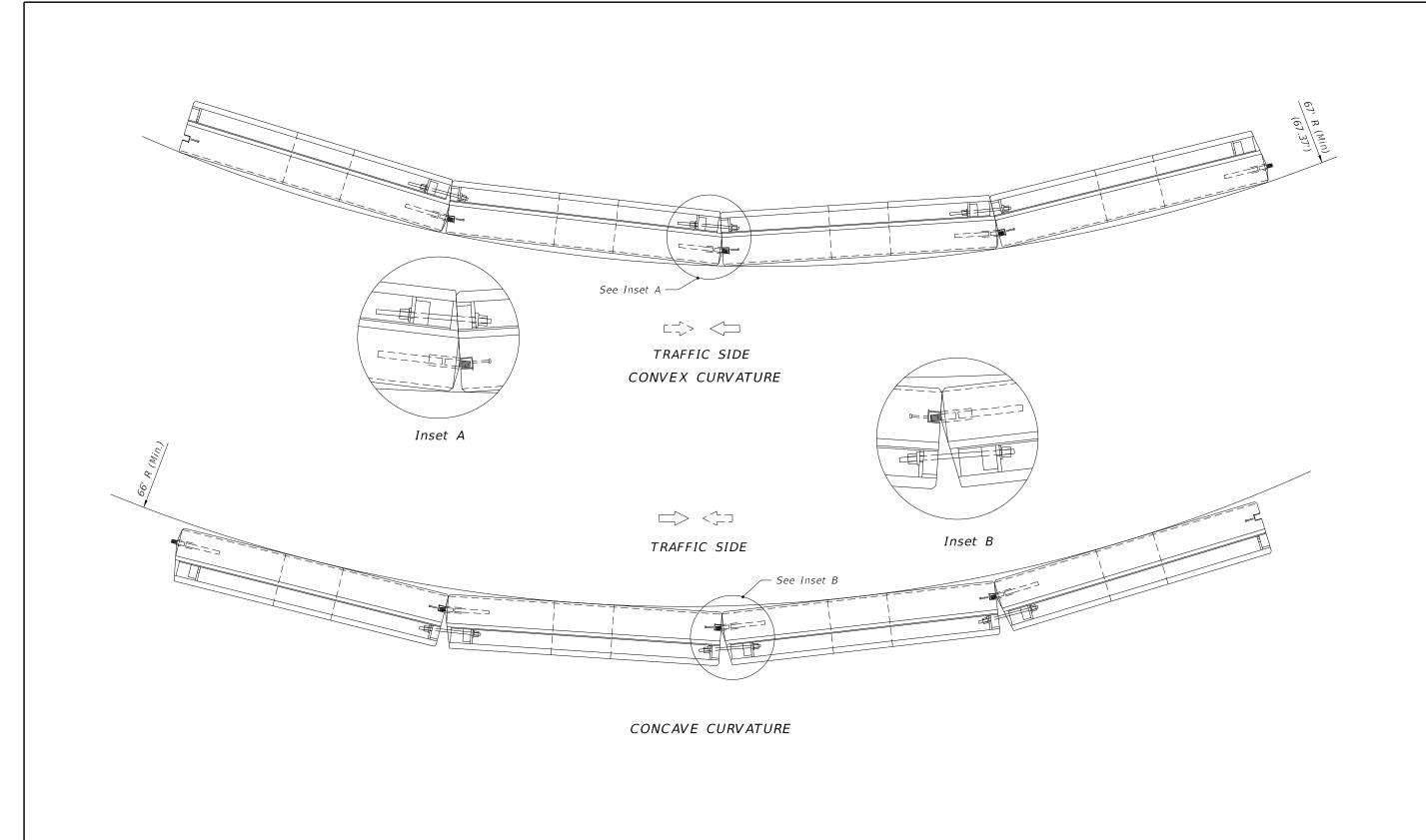
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LOW PROFILE BARRIER

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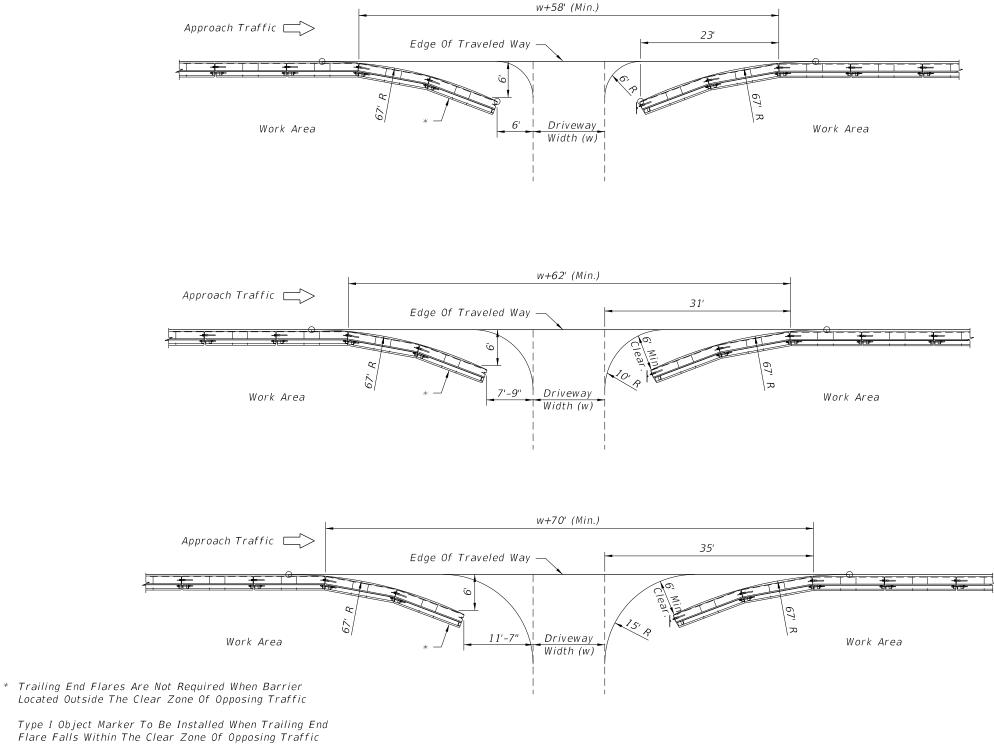
MAXIMUM CURVATURE ● MINIMUM RADIUS PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

≥ DESCRIPTION: REVISION 11/01/17

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Type I Object Marker To Be Installed When Trailing End Flare Falls Within The Clear Zone Of Opposing Traffic

LEGEND

BARRIER OPENINGS AT DRIVEWAYS

Type I Object Marker

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

REVISION 11/01/17

DESCRIPTION:

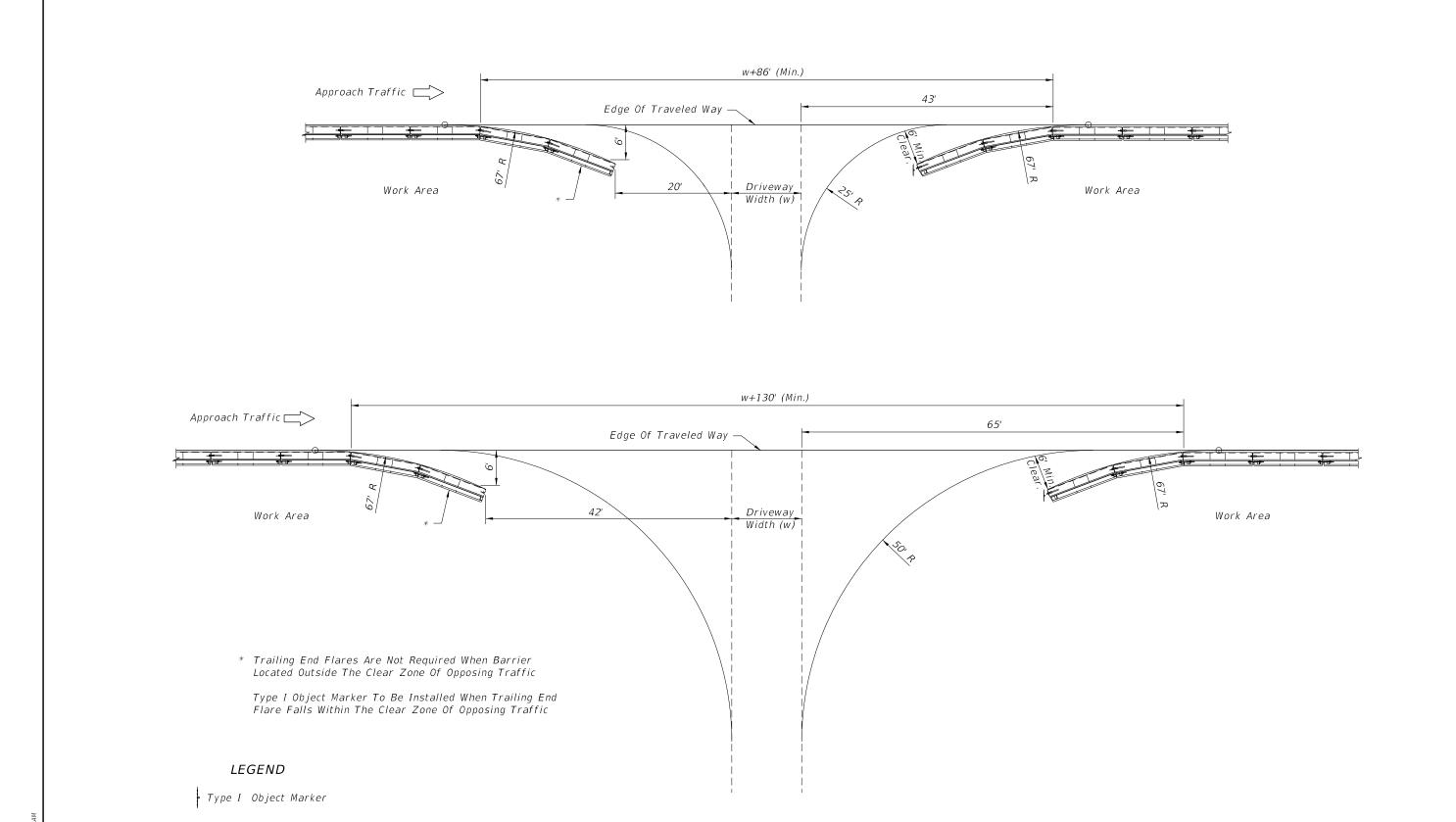
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LOW PROFILE BARRIER

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BARRIER OPENINGS AT DRIVEWAYS

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

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≥ DESCRIPTION:

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LOW PROFILE BARRIER

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GENERAL NOTES:

- 1. All projects and works on highways, roads and streets shall have a traffic control plan. All work shall be executed under the established plan and Department-approved procedures. This Index contains information specific to the Federal and State guidelines and standards for the preparation of traffic control plans and for the execution of traffic control in work zones, for construction and maintenance operations and utility work on highways, roads and streets on the State Highway System. Certain requirements in this Index are based on the high volume nature of State Highways. For highways, roads and streets off the State Highway System, the local agency (City/County) having jurisdiction may adopt requirements based on the minimum requirements provided in the MUTCD.
- 2. Indexes 102-601 through 102-670 are Department-specific typical applications of commonly encountered situations. Adjust device location or number thereof as recommended by the Worksite Traffic Supervisor and approved by the Engineer. Devices include, but are not limited to, Flaggers, portable temporary signals, signs, pavement markings, and channelizing devices. Comply with MUTCD or applicable Department criteria for any changes and document the reason for the change.
- 3. Except for emergencies, any road closure on State Highway System shall comply with Section 335.15, F.S.

≥ DESCRIPTION:

REVISION

11/01/17

DEFINITIONS

Regulatory Speed (In Work Zones)

The maximum permitted travel speed posted for the work zone is indicated by the regulatory speed limit signs. The work zone speed must be shown or noted in the plans. This speed should be used as the minimum design speed to determine runout lengths, departure rates, flare rates, lengths of need, clear zone widths, taper lengths, crash cushion requirements, marker spacings, superelevation and other similar features.

Advisory Speed

The maximum recommended travel speed through a curve or a hazardous area.

Travel Way

The portion of the roadway for the movement of vehicles. For traffic control through work zones, travel way may include the temporary use of shoulders and any other permanent or temporary surface intended for use as a lane for the movement of vehicular traffic.

- a. Travel Lane: The designated widths of roadway pavement marked to carry through traffic and to separate it from opposing traffic or traffic occupying other traffic lanes.
- b. Auxiliary Lane: The designated widths of roadway pavement marked to separate speed change, turning, passing and climbing maneuvers from through traffic.

Detour, Lane Shift, and Diversion

A detour is the redirection of traffic onto another roadway to bypass the temporary traffic control zone. A lane shift is the redirection of traffic onto a different section of the permanent pavement. A diversion is the redirection of traffic onto a temporary roadway, usually adjacent to the permanent roadway and within the limits of the right of way.

Aboveground Hazard

An aboveground hazard is any object, material or equipment other than traffic control devices that encroaches upon the travel way or that is located within the clear zone which does not meet the Department's safety criteria, i.e., anything that is greater than 4" in height and is firm and unyielding or doesn't meet breakaway requirements.

TEMPORARY TRAFFIC CONTROL DEVICES

All temporary traffic control devices shall be ON the Department's Approved Products List (APL). Ensure the appropriate APL number is permanently marked on the device in a readily visible location.

All temporary traffic control devices shall be removed as soon as practical when they are no longer needed. When work is suspended for short periods of time, temporary traffic control devices that are no longer appropriate shall be removed or covered.

Arrow Boards, Portable Changeable Message Signs, Radar Speed Display Trailer, Portable Regulatory Signs, and any other trailer mounted device shall be delineated with a channelizing device placed at each corner when in use and shall be moved outside the travel way and clear zone or be shielded by a barrier or crash cushion when not in use.

PEDESTRIAN AND BICYCLIST

When an existing pedestrian way or bicycle way is located within a traffic control work zone, accommodation must be maintained and provision for the disabled must be provided.

Only approved pedestrian longitudinal channelizing devices may be used to delineate a temporary traffic control zone pedestrian walkway.

Advanced notification of sidewalk closures and marked detours shall be provided by appropriate signs.

OVERHEAD WORK

Work is only allowed over a traffic lane when one of the following ontions is used:

OPTION 1 (OVERHEAD WORK USING A MODIFIED LANE CLOSURE)

Overhead work using a modified lane closure is allowed if all of the following conditions are met:

- a. Work operation is located in a signalized intersection and
 limited to signals, signs, lighting and utilities.
- b. Work operations are 60 minutes or less.
- c. Speed limit is 45 mph or less.
- d. Aerial lift equipment in the work area has high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- e. Aerial lift equipment is placed directly below the work area to close the lane.
- f. Traffic control devices are placed in advance of the vehicle/equipment closing the lane using a minimum 100 foot taper.
- g. Volume or complexity of the roadway may dictate additional devices, signs, flagmen and/or a traffic control officer.

OPTION 2 (OVERHEAD WORK ABOVE AN OPEN TRAFFIC LANE)

Overhead work above a open traffic lane is allowed if all of the following conditions are met:

- a. Work operation is located on a utility pole, light pole, signal pole, or their appurtenances.
- b. Work operations are 60 minutes or less.
- c. Speed limit is 45 mph or less.
- d. No encroachment by any part of the work activities and equipment within an area bounded by 2 feet outside the edge of travel way and 18 feet high.
- e. Aerial lift equipment in the work area has high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- f. Volume or complexity of the roadway may dictate additional devices, signs, flagmen and/or a traffic control officer.
- g. Adequate precautions are taken to prevent parts, tools, equipment and other objects from falling into open lanes of traffic.
- h. Other Governmental Agencies, Rail facilities, or Codes may require a greater clearance. The greater clearance required prevails as the rule.

OPTION 3 (OVERHEAD WORK ADJACENT TO AN OPEN TRAFFIC LANE)

Overhead work adjacent to an open traffic lane is allowed if all of the following conditions are met:

- a. Work operation is located on a utility pole, light pole, signal pole, or their appurtenances.
- b. Work operations are 1 day or less.
- c. Speed limit is 45 mph or less.
- d. No encroachment by any part of the work activities and equipment within 2 foot from the edge of travel way up to 18' height.
 - Above 18' in height, no encroachment by any part of the work activities and equipment over the open traffic lane (except as allowed in Option 2 for work operations of 60 minutes or less).
- e. Aerial lift equipment in the work area has high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- f. Volume or complexity of the roadway may dictate additional devices, signs, flagmen and/or a traffic control officer.
- g. Adequate precautions are taken to prevent parts, tools, equipment and other objects from falling into open lanes of traffic.
- h. Other Governmental Agencies, Rail facilities, or Codes may require a greater clearance. The greater clearance required prevails as the rule.

OPTION 4 (OVERHEAD WORK MAINTAINING TRAFFIC WITH NO ENCROACHMENT BELOW THE OVERHEAD WORK AREA)

Traffic shall be detoured, shifted, diverted or paced as to not encroach in the area directly below the overhead work operations in accordance with the appropriate index drawing or detailed in the plans. This option applies to, but not limited to, the following construction activities:

- a. Beam, girder, segment, and bent/pier cap placement.
- b. Form and falsework placement and removal.
- c. Concrete placement.
- d. Railing construction located at edge of deck.
- e. Structure demolition.

OPTION 5 (CONDUCTOR/CABLE PULLING ABOVE AN OPEN TRAFFIC LANE)

Overhead cable and/or de-energized conductor installations initial pull to proper tension shall be done in accordance with the appropriate Index or temporary traffic control plan.

Continuous pulling operations of secured cable and/or conductors are allowed over open lane(s) of traffic with no encroachment by any part of the work activities, materials or equipment within the minimal vertical clearance above the travel way. The utility shall take precautions to ensure that pull ropes and conductors/cables at no time fall below the minimum vertical clearance.

On Limited Access facilities, a site specific temporary traffic control plan is required. The temporary traffic control plan shall include:

- a. The temporary traffic control set up for the initial pulling of the pull rope across the roadway.
- b. During pulling operations, advance warning consisting of no less than a Changeable Message Sign upstream of the work area with alternating messages, "Overhead Work Ahead" and "Be Prepared to Stop" followed by a traffic control officer and police vehicle with blue lights flashing during the pulling operation.

RAILROADS

Railroad crossings affected by a construction project should be evaluated for traffic controls to reduce queuing on the tracks. The evaluation should include as a minimum: traffic volumes, distance from the tracks to the intersections, lane closure or taper locations, signal timing, etc.

SIGHT DISTANCE

Tapers: Transition tapers should be obvious to drivers. If restricted sight distance is a problem (e.g., a sharp vertical or horizontal curve), the taper should begin well in advance of the view obstruction. The beginning of tapers should not be hidden behind curves.

Intersections: Traffic control devices at intersections must provide sight distances for the road user to perceive potential conflicts and to traverse the intersection safely. Construction equipment and materials shall not restrict intersection sight distance.

ABOVEGROUND HAZARD

Aboveground hazards (see definitions) are to be considered work areas during working hours and treated with appropriate work zone traffic control procedures. During nonworking hours, all objects, materials and equipment that constitute an aboveground hazard must be stored/placed outside the travel way and clear zone or be shielded by a barrier or crash cushion.

For aboveground hazards within a work zone the clear zone required should be based on the regulatory speed posted during construction.

DESCRIPTION:



FY 2019-20 STANDARD PLANS

102-600

CLEAR ZONE WIDTHS FOR WORK ZONES

The term 'clear zone' describes the unobstructed relatively flat area, impacted by construction, extending outward from the edge of the traffic lane. The table below gives clear zone widths in work zones for medians and roadside conditions other than for roadside canals; where roadside canals are present, clear zone widths are to conform with the distances to canals as described in the FDOT Design Manual 215.2.

CLEAR ZONE WIDTHS FOR WORK ZONES						
WORK ZONE SPEED (MPH)	TRAVEL LANES & MULTILANE RAMPS (feet)	AUXILIARY LANES & SINGLE LANE RAMPS (feet)				
60-70	30	18				
55	24	14				
45-50	18	10				
30-40	14	10				
ALL SPEEDS CURB & GUTTER	4' BEHIND FACE OF CURB	4' BEHIND FACE OF CURB				

SUPERELEVATION

Horizontal curves constructed in conjunction with work zone traffic control should have the required superelevation applied to the design radii. Under conditions where normal crown controls curvature, the minimum radii that can be applied are listed in the table below.

MINIMUM RADII FOR				
NORMAL CROWN				
WORK ZONE POSTED SPEED	MINIMUM RADIUS			
MPH	feet			
70	4090			
65	3130			
60	2400			
55	1840			
50	1390			
45	1080			
40	820			
35	610			
30	430			
Superelevate When Smaller				
Radii is Used				

OVERWEIGHT/OVERSIZE VEHICLES

Restrictions to Lane Widths, Heights or Load Capacity can greatly impact the movement of over dimensioned loads. The Contractor shall notify the Engineer who in turn shall notify the State Permits Office, phone no. (850) 410-5777, at least seven calendar days in advance of implementing a maintenance of traffic plan which will impact the flow of overweight/oversized vehicles. Information provided shall include location, type of restriction (height, width or weight) and restriction time frames. When the roadway is restored to normal service the State Permits Office shall be notified immediately.

LANE WIDTHS

Lane widths of through roadways should be maintained through work zone travel ways wherever practical. The minimum widths for work zone travel lanes shall be as follows: 11' for Interstate with at least one 12' lane provided in each direction, unless formally excepted by the Federal Highway Administration; 11' for freeways; and 10' for all other facilities.

HIGH-VISIBILITY SAFETY APPAREL

All high-visibility safety apparel shall meet the requirements of the International Safety Equipment Association (ISEA) and the American National Standards Institute (ANSI) for "High-Visibility Safety Apparel", and labeled as ANSI/ISEA 107-2004 or newer. The apparel background (outer) material color shall be either fluorescent orange-red or fluorescent yellow-green as defined by the standard. The retroreflective material shall be orange, yellow, white, silver, yellow-green, or a fluorescent version of these colors, and shall be visible at a minimum distance of 1,000 feet. Class 3 apparel may be substituted for Class 2 apparel. Replace apparel that is not visible at 1,000 feet.

WORKERS: All workers within the right-of-way shall wear ANSI/ISEA Class 2 apparel. Workers operating machinery or equipment in which loose clothing could become entangled during operation shall wear fitted high-visibility safety apparel. Workers inside the bucket of a bucket truck are not required to wear high-visibility safety apparel.

UTILITIES: When other industry apparel safety standards require utility workers to wear apparel that is inconsistent with FDOT requirements such as NFPA, OSHA, ANSI, etc., the other standards for apparel may prevail.

FLAGGERS: For daytime activities, Flaggers shall wear ANSI/ISEA Class 2 apparel. For nighttime activities, Flaggers shall wear ANSI/ISEA Class 3 apparel.

REGULATORY SPEEDS IN WORK ZONES

Traffic Control Plans (TCP's) for all projects must include specific regulatory speeds for each phase of work. This can either be the posted speed or a reduced speed. The speed shall be noted in the TCPs; this includes indicating the existing speed if no reduction is to be made. Regulatory speeds are to be uniformly established through each phase.

In general, the regulatory speed should be established to route vehicles safely through the work zone as close as to normal highway speed as possible. The regulatory speed should not be reduced more than 10 mph below the posted speed and never below the minimum statutory speed for the class of facility. When a speed reduction greater than 10 mph is imposed, the reduction is to be done in 10 mph per 500' increments.

Temporary regulatory speed signs shall be removed as soon as the conditions requiring the reduced speed no longer exist. Once the work zone regulatory speeds are removed, the regulatory speed existing prior to construction will automatically go back into effect unless new speed limit signing is provided for in the plans.

On projects with interspaced work activities, speed reductions should be located in proximity to those activities which merit a reduced speed, and not "blanketed" for the entire project. At the departure of such activities, the normal highway speed should be posted to give the motorist notice that normal speed can be resumed.

If the existing regulatory speed is to be used, consideration should be given to supplementing the existing signs when the construction work zone is between existing regulatory speed signs. For projects where the reduced speed conditions exist for greater than 1 mile in rural areas (non-interstate) and on rural or urban interstate, additional regulatory speed signs are to be placed at no more than 1 mile intervals. Engineering judgement should be used in placement of the additional signs. Locating these signs beyond ramp entrances and beyond major intersections are examples of proper placement. For urban situations (non-interstate), additional speed signs are to be placed at a maximum of 1000' apart.

When field conditions warrant speed reductions different from those shown in the TCP the contractor may submit to the project engineer for approval by the Department, a signed and sealed study to justify the need for further reducing the posted speed, or, the engineer may request the District Traffic Operations Engineer (DTOE) to investigate the need. It will not be necessary for the DTOE to issue regulations for regulatory speeds in work zones due to the revised provisions of F.S. 316.07451(2) (b). Advisory Speed plates will be used at the option of the field engineer for temporary use while processing a request to change the regulatory speed specified in the plans when deemed necessary. Advisory speed plates cannot be used alone but must be placed below the construction warning sign for which the advisory speed is required.

For additional information, refer to the FDOT Design Manual 240.

LENGTH OF LANE CLOSURES

For interstates and state highways with a posted speed of 55MPH or greater, lane closures must not exceed 3 miles (includes taper, buffer, and work zone) in any given direction and must not close two consecutive interchanges.

The flagger must be clearly visible to approaching traffic for a distance sufficient to permit proper response by the motorist to the flagging instructions, and to permit traffic to reduce speed or to stop as required before entering the work site. Flaggers shall be positioned to maintain maximum color contrast between the Flagger's high-visibility safety apparel and equipment and the work area background.

Hand-Signaling Devices

STOP/SLOW paddles are the primary hand-signaling device. The STOP/SLOW paddle shall have an octagonal shape on a rigid handle. If the STOP/SLOW paddle is placed on a rigid staff, the minimum length of the staff, measured from the bottom of the paddle to the end of the staff that rests on the ground, must not be less than 6 ft. STOP/SLOW paddles shall be at least 24 inches wide with letters at least 6 inches high and should be fabricated from light semirigid material. The background of the STOP face shall be red with white letters and border. The background of the SLOW face shall be orange with black letters and border. When used at night-time, the STOP/SLOW paddle shall be retroreflectorized.

Flag use is limited to immediate emergencies, intersections, and when working on the centerline or shared left turn lanes where two (2) flaggers are required and there is opposing traffic in the adjacent lanes. Flags, when used, shall be a minimum of 24 inches square, made of a good grade of red material, and securely fastened to a staff that is approximately 36 inches in length. When used at nighttime, flags shall be retroreflectorized red.

Flashlight, lantern or other lighted signal that will display a red warning light shall be used at night.

Flagger Stations

Flagger stations shall be located far enough in advance of the work space so that approaching road users will have sufficient distance to stop before entering the work space. When used at nighttime, the flagger station shall be illuminated.

SURVEY WORK ZONES

The SURVEY CREW AHEAD symbol or legend sign shall be the principal Advance Warning Sign used for Traffic Control Through Survey Work Zones and may replace the ROAD WORK AHEAD sign when lane closures occur, at the discretion of the Party Chief.

When Traffic Control Through Work Zones is being used for survey purposes only, the END ROAD WORK sign as called for on certain 102 Series of Indexes should be omitted.

Survey Between Active Traffic Lanes or Shared Left Turn Lanes

The following provisions apply to Main Roadway Traffic Control Work Zones. These provisions must be adjusted by the Party Chief to fit roadway and traffic conditions when the Survey Work Zone includes intersections.

- (A) A STAY IN YOUR LANE (MOT-1-06) sign shall be added to the Advance Warning Sign sequence as the second most immediate sign from the work area.
- (B) Elevation Surveys-Cones may be used at the discretion of the Party Chief to protect prism holder and flagger(s). Cones, if used, may be placed at up to 50' intervals along the break line throughout the work zone.
- (C) Horizontal Control-With traffic flow in the same direction, cones shall be used to protect the backsight tripod and/or instrument. Cones shall be placed at the equipment, and up to 50' intervals for at least 200' towards the flow of traffic.
- (D) Horizontal Control-With traffic flow in opposite directions, cones shall be used to protect the backsight tripod and/or instrument. Cones shall be placed at the equipment, and up to 50' intervals for at least 200' in both directions towards the flow of traffic.

SIGNS

SIGN MATERIALS

Mesh signs and non-retroreflectice vinyl signs may only be used for daylight operations. Non-retroreflectice vinyl signs must meet the requirements of Specifications Section 994.

Retroreflective vinyl signs meeting the requirements of Specification Section 994 may be used for daylight or night operations not to exceed 1 day except as noted in the Indexes.

Rigid or Lightweight sign panels may be used in accordance with the vendor APL drawing for the sign stand to which they are attached.

INTERSECTING ROAD SIGNING

Signing for the control of traffic entering and leaving work zones by way of intersecting crossroads shall be adequate to make drivers aware of work zone conditions. When Work operations exceed 60 minutes, place the ROAD WORK AHEAD sign on the side street entering the work zone.

ADJOINING AND/OR OVERLAPPING WORK ZONE SIGNING

Adjoining work zones may not have sufficient spacing for standard placement of signs and other traffic control devices in their advance warning areas or in some cases other areas within their traffic control zones. Where such restraints or conflicts occur or are likely to occur, one of the following methods will be employed to avoid conflicts and prevent conditions that could lead to misunderstanding on the part of the traveling public as to the intended travel way by the traffic control procedure applied:

- (A) For scheduled projects the engineer in responsible charge of project design will resolve anticipated work zone conflicts during the development of the project traffic control plan. This may entail revision of plans on preceding projects and coordination of plans on concurrent projects.
- (B) Unanticipated conflicts arising between adjoining in progress highway construction projects will be resolved by the Resident Engineer for projects under his residency, and, by the District Construction Engineer for in progress projects under adjoining residencies.
- (C) The District Maintenance Engineer will resolve anticipated and occurring conflicts within scheduled maintenance operations.
- (D) The Unit Maintenance Engineer will resolve conflicts that occur within routine maintenance works; between routine maintenance work, unscheduled work and/or permitted work; and, between unit controlled maintenance works and highway construction projects.

SIGN COVERING AND INTERMITTENT WORK STOPPAGE SIGNING

Existing or temporary traffic control signs that are no longer applicable or are inconsistent with intended travel paths shall be removed or fully covered.

Sign blanks or other available coverings must completely cover the existing sign. Rigid sign coverings shall be the same size as the sign it is covering, and bolted in a manner to prevent movement

Sign covers are incidental to work operations and are not paid for separately.

SIGNING FOR DETOURS, LANE SHIFTS AND DIVERSIONS

Detours should be signed clearly over their entire length so that motorists can easily determine how to return to the original roadway. The reverse curve (W1-4) warning sign should be used for the advanced warning for a lane shift. A diversion should be signed as a lane shift.

EXTENDED DISTANCE ADVANCE WARNING SIGN

Advance Warning Signs shall be used at extended distance of one-half mile or more when limited sight distance or the nature of the obstruction may require a motorist to bring their vehicle to a stop. Extended distance Advanced Warning Signs may be required on any type roadway, but particularly be considered on multilane divided highways where vehicle speed is generally in the higher range (45 MPH or more).

UTILITY WORK AHEAD SIGN

The UTILITY WORK AHEAD (W21-7) sign may be used as an alternate to the ROAD WORK AHEAD or the ROAD WORK XX FT (W20-1) sign for utility operations on or adjacent to a highway.

LENGTH OF ROAD WORK SIGN

The length of road work sign (G20-1) bearing the legend ROAD WORK NEXT_____ MILES is required for all projects of more than 2 miles in length. The number of miles entered should be rounded up to the nearest mile. The sign shall be located at begin construction points.

SPEEDING FINES DOUBLED WHEN WORKERS PRESENT SIGN

The SPEEDING FINES DOUBLED WHEN WORKERS PRESENT sign should be installed on all projects, but may be omitted if the work operation is less than 1 day. The placement should be 500 feet beyond the ROAD WORK AHEAD sign or midway to the next sign whichever is less.

GROOVED PAVEMENT AHEAD SIGN

The GROOVED PAVEMENT AHEAD sign is required 500 feet in advance of a milled or grooved surface open to traffic. The W8-15P placard shall be used in conjunction with the GROOVED PAVEMENT AHEAD sign.

END ROAD WORK SIGN

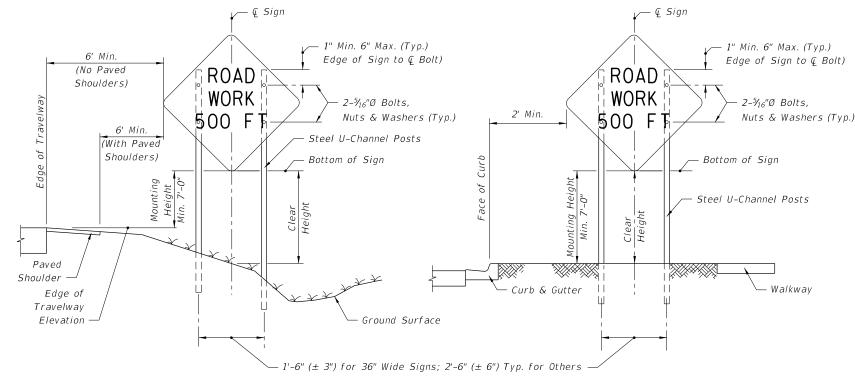
The END ROAD WORK sign (G20–2) should be installed on all projects, but may be omitted where the work operation is less than 1 day. The sign should be placed approximately 500 feet beyond the end of a construction or maintenance project unless other distance is called for in the plans. When other Construction or Maintenance Operations occur within 1 mile this sign should be omitted and signing coordinated in accordance with Index 102–600. ADJOINING AND/OR OVERLAPPING WORK ZONE SIGNING.

PROJECT INFORMATION SIGN

The Project information sign shall be installed when called for in the plans.

- a. Road closure signs mounted in accordance with the vendor drawing for the Type III Barricade shown on the APL.
- b. Pedestrian advanced warning or pedestrian regulatory signs mounted on sign supports in accordance with the vendor drawing shown on the APL.
- c. Median barrier mounted signs per Index 700-013.
- 2. Unless shielded with barrier or outside of the Clear Zone, signs mounted on temporary supports or barricades, and barricade/sign combination must be crashworthy in accordance with NCHRP 350 requirements and included on the Approved Products List (APL).
- 3. Use only approved systems listed on the Department's Approved Products List (APL).
- 4. Manufacturers seeking approval of U-Channel and steel square tube sign support assemblies for inclusion on the Approved Products List (APL) must submit a APL application, design calculations (for square tube only), and detailed drawings showing the product meets all the requirements of this Index.
- 5. Provide 3 lb/ft Steel U-Channel Posts with a minimum section modulus of 0.43 in³ for 60 ksi steel, a minimum section modulus of 0.37 in³ for 70 ksi steel, or a minimum section modulus of 0.34 in³ for 80 ksi steel.
- 6. Provide 4 lb/ft Steel U-Channel Posts with a minimum section modulus of 0.56 in³ for 60 ksi steel. or a minimum section modulus of 0.47 in³ for 70 ksi or 80 ksi steel.
- 7. U-channel posts shall conform with ASTM A 499, Grade 60, or ASTM A 576, Grade 1080 (with a minimum yield strength of 60 ksi). Square tube posts shall conform with ASTM A 653, Grade 50, or ASTM A 1011, Grade 50.
- 8. Sign attachment bolts, washers, nuts, and spacers shall conform with ASTM A307 or A 36.
- 9. For diamond warning signs with supplement plaque (up to 5 ft² in area), use 4 lb/ft posts for up to 10 ft Clear Height (measure to the bottom of diamond warning sign).
- 10. Install 4 lb/ft Steel U-Channel Posts with approved breakaway splice in accordance with the manufacturer's detail shown on the APL.
- 11. The contractor may install 3 lb/ft Steel U-Channel Posts with approved breakaway splice in accordance with the manufacturer's detail shown on the APL.
- 12. Install all posts plumb.
- 13. The contractor may set posts in preformed holes to the specified depth with suitable backfill tamped securely on all sides, or drive 3 lb/ft sign posts and any size base post in accordance with the manufacturer's detail shown on the APL

DESCRIPTION:



2 POST SIGN SUPPORT MOUNTING DETAILS (SINGLE POST SIMILAR) RURAL

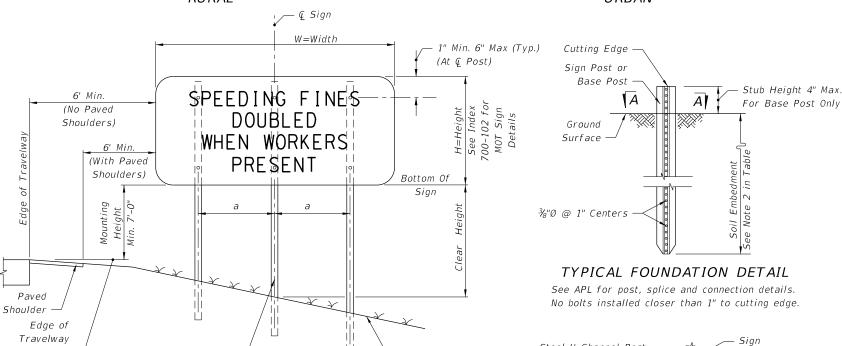
2 POST SIGN SUPPORT MOUNTING DETAILS (SINGLE POST SIMILAR) URBAN

Steel U-Channel Post

Lock Washer

(¾₁₆" Nominal Size)

5/16" Steel Hex Nut



Ground Surface

3 POST SIGN SUPPORT MOUNTING DETAILS

Where W = 48'': $a = 1' - 4\frac{1}{2}'' (\pm 1'')$ W = 60'': $a = 1' - 9'' (\pm 1'')$ W = 72'': $a = 2' - 1'' (\pm 1'')$

Steel U-Channel Posts

SECTION A-A (SCHEMATIC) SIGN ATTACHMENT DETAIL (WITHOUT Z-BRACKET)

5/16" Steel Hex

Head Bolt

Flat Washer

(5/16" Nominal Size)

POST AND FOUNDATION TABLE FOR WORK ZONE SIGNS

SIGN SIZE | NUMBER OF STEEL SIGN SHAPE (inches) U CHANNEL POSTS Octagon 30x30 36x36x36 Triangle 48x48x48 60x60x60 24×18 24x30 30x24 36x18 36x24 48 x 18 Rectangle 48x24 $(W \times H)$ 36 x 48 48x30 48x36 54x36 48x60 60x54 72x48 120x60* 30x30 Square 36 x 36 48x48 Diamond 48×48 2 (See Note 7) Circle 36Ø 2

Notes For Table:

- 1. Use 3 lb/ft posts for Clear Height up to 10' and 4 lb/ft posts for Clear Height up to 12'.
- Use 4 lb/ft U-channel sign post with a mounting height of 7' min. and 8' max. Attach sign panel using Z-bracket detail on Sheet 6.
- 2. Minimum foundation depth is 4.0' for 3 lb/ft posts and 4.5' for 4 lb/ft posts.
- 3. For both 3 lb/ft and 4 lb/ft base or sign posts installed in rock, a minimum cumulative depth of 2' of rock layer is required.
- 4. The soil plate as shown on the APL vendor drawing is not required for base posts or sign posts installed in existing rock (as defined in Note 3), asphalt roadway, shoulder pavement or soil under sidewalk.

WORK ZONE SIGN SUPPORTS

REVISION 11/01/18

FDOT

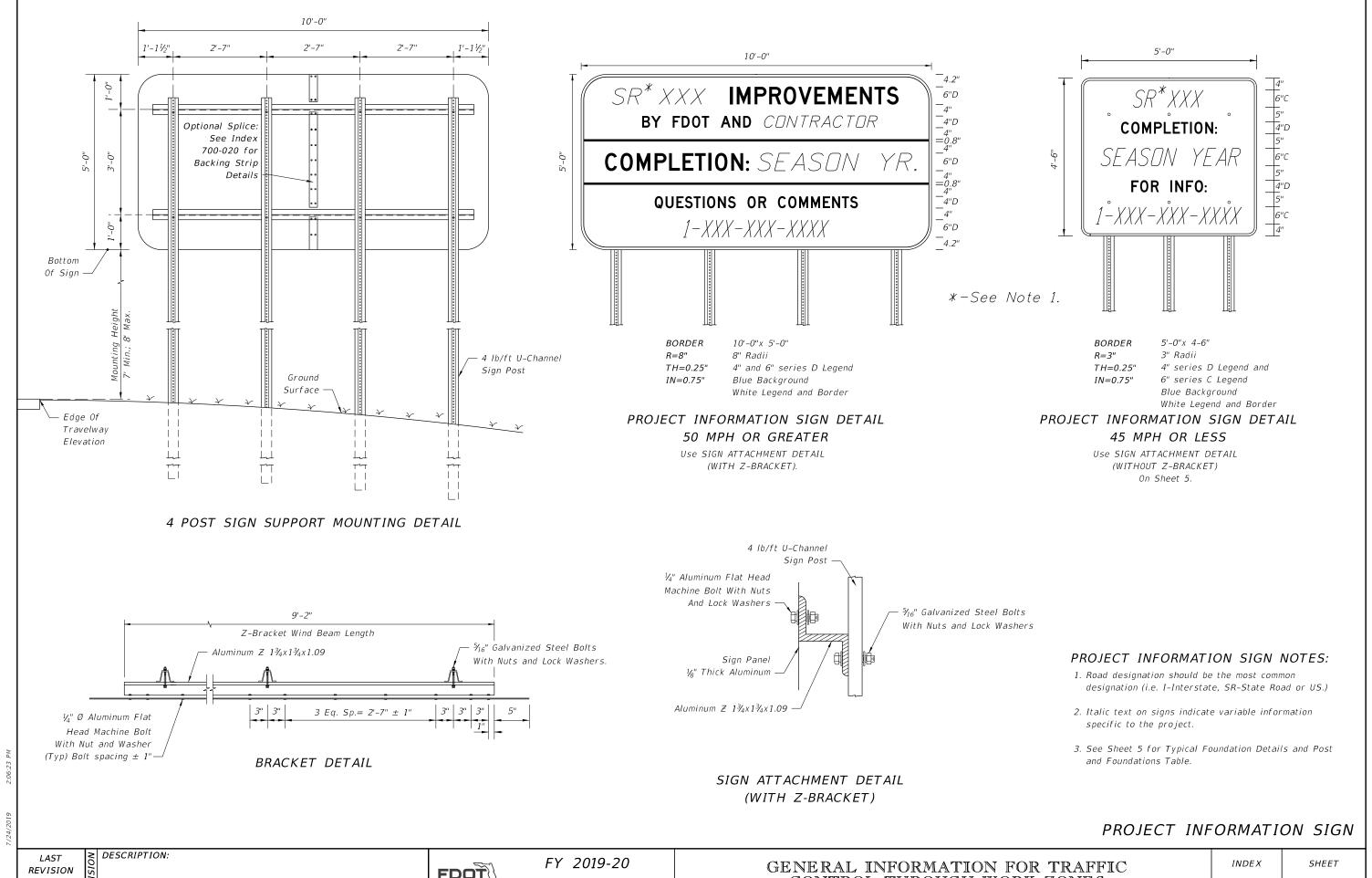
Elevation

FY 2019-20 STANDARD PLANS

GENERAL INFORMATION FOR TRAFFIC CONTROL THROUGH WORK ZONES

INDEX 102-600

SHEET



11/01/17

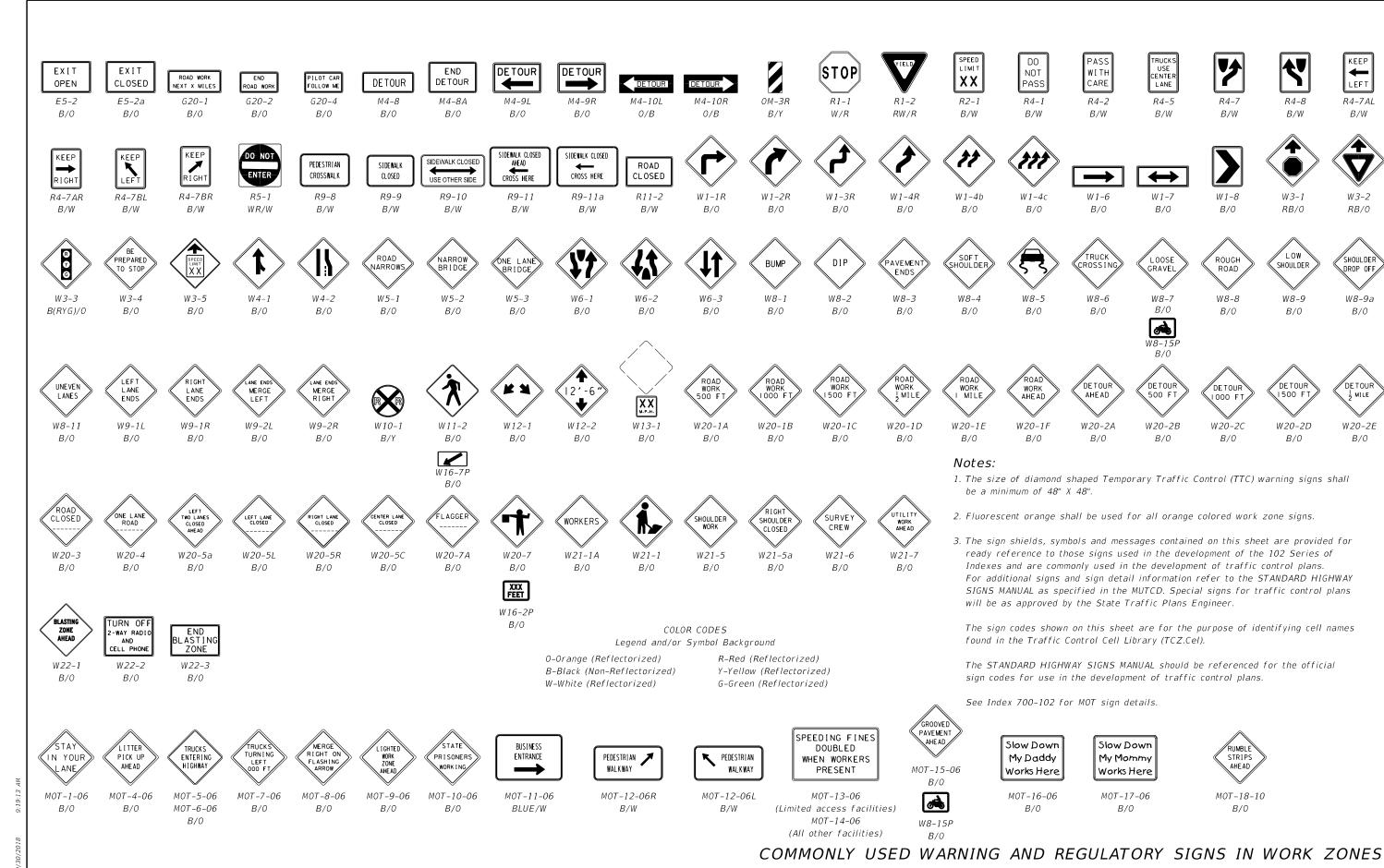
FDOT

STANDARD PLANS

CONTROL THROUGH WORK ZONES

102-600

6 of 12



KEEP

←

LEFT

R4-7AL

B/W

W3-2

RB/0

SHOULDER DROP OFF

W8-9a

B/0

DETOUR

MILE

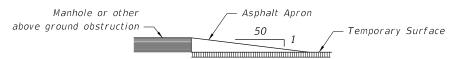
W20-2E

B/0

MANHOLES/CROSSWALKS/JOINTS

Manholes extending 1" or more above the travel lane and crosswalks having an uneven surface greater than $\frac{1}{4}$ " shall have a temporary asphalt apron constructed as shown in the diagram below.

All transverse joints that have a difference in elevation of 1" or more shall have a temporary asphalt apron constructed as shown in the diagram below.



The apron is to be removed prior to constructing the next lift of asphalt. The cost of the temporary asphalt shall be included in the contract unit price for Maintenance of Traffic, LS.

REMOVING PAVEMENT MARKINGS

Existing pavement markings that conflict with temporary work zone delineation shall be removed by any method approved by the Engineer, where operations exceed one daylight period. Remove conflicting pavement marking using a method that will not damage the surface texture of the pavement, unless the pavement will be restored prior to traffic use. Painting over existing pavement markings with black paint or spraying with asphalt shall not be accepted as substitute for removal or obliteration. Full pavement width overlays of either a structural or friction course (non-final surface) are an acceptable alternate means to achieve removal.

SIGNALS

Existing traffic signal operations that require modification in order to carry out work zone traffic control shall be included in the TCP and be approved by the District Traffic Operations

Maintain all existing actuated or traffic responsive mode signal operations for main and side street movements for the duration of the Contract and require restoration of any loss of detection within 12 hours. The contractor shall select only detection technology listed on the Department's Approved Products List (APL) and approved by the Engineer to restore detection capabilities.

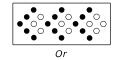
ADVANCE WARNING ARROW BOARDS

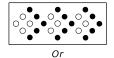
An arrow board in the arrow or chevron mode shall be used only for stationary or moving lane closures on multilane roadways.

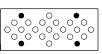
For shoulder work, blocking the shoulder, for roadside work near the shoulder, or for temporarily closing one lane on a two-lane, two-way roadway, an arrow board shall be used only in the caution mode.

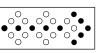
A single arrow board shall not be used to merge traffic laterally more than one lane. When arrow boards are used to close multiple lanes, a single board shall be used at the merging taper for each closed lane.

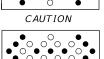
When Advance Warning Arrow Boards are used at night, the intensity of the flashers shall be reduced during darkness when lower intensities are desirable.











MOVE/MERGE LEFT

MOVE/MERGE RIGHT

MOVE/MERGE RIGHT OR LEFT

- Minimum Required Lamps
- Additional Lamps Allowed

MODES

PORTABLE CHANGEABLE MESSAGE SIGNS (PCMS)

The PCMS can be used to:

- 1. Supplement standard signing in construction or maintenance work zones
- 2. Reinforce static advance warning messages.
- 3 Provide motorists with updated guidance information.

PCMS should be placed approx, 500 to 800 feet in advance of the work zone conflicts or 0.5 to 2 miles in advance of complex traffic control schemes which require new and/or unusual traffic maneuvers.

If PCMS are to be used at night, the intensity of the flashers shall be reduced during darkness when lower intensities are desirable.

For additional information refer to the FDOT Design Manual 240.

TRUCK/TRAILER-MOUNTED ATTENUATORS

Truck/Trailer-mounted attenuators (TMA) can be used for moving operations and short-term stationary operations. For moving operations, see Indexes 102-607 and 102-619. For short-term, stationary operations, see Part VI of the MUTCD.

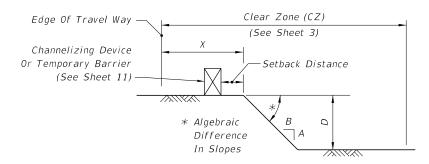
CHANNELIZING DEVICES

Channelizing devices for work zone traffic control shall be as prescribed in Part VI of the MUTCD, subject to supplemental revisions provided in the contract documents and the 102 Series of Indexes. Lighting Devices must not be used to supplement channelization.

CHANNELIZING DEVICE CONSISTENCY

Barricades, vertical panels, cones, tubular markers and drums shall not be intermixed within either the lateral transition or within the tangent alignment.

- 3. Drop-offs may be mitigated by placement of slopes with optional base material per Specification 285. Slopes shallower than 1:4 may be required to avoid algebraic difference in slopes greater than 0.25. Include the cost for the placement and removal of the material in Maintenance of Traffic, LSD. Use of this treatment in lieu of a temporary barrier is not eligible for CSIP consideration. Conduct daily inspections for deficiencies related to erosion, excessive slopes, rutting or other adverse conditions. Repair any deficiencies immediately.
- 4. For Setback Distance, refer to the Index or Approved Products List (APL) drawing of
- 5. For Conditions 1 and 3 provided in Table 1, any drop-off condition that is created and restored within the same work period will not be subject to the use of temporary barriers; however, channelizing devices will be required.
- 6. When permanent curb heights are \geq 6", no channelizing device will be required. For curb heights < 6", see Table 1.

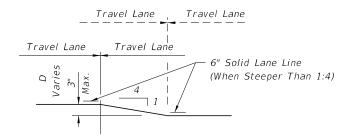


DROP-OFF CONDITION DETAIL

Table 1 Drop-off Protection Requirements					
Condition	X (ft)	D (in.)	Device Required		
1	0-12	> 3	Temporary Barrier		
2	> 12-CZ	> 3 to ≤ 5	Channelizing Device		
3	0-CZ	> 5	Temporary Barrier		
4	Removal of Bridge or Retaining Wall Barrier		Temporary Barrier		
5	Removal of portions of Bridge Deck		Temporary Barrier		

TRAVEL LANE TREATMENT FOR MILLING OR RESURFACING NOTES

- 1. This treatment applies to resurfacing or milling operations between adjacent travel lanes.
- 2. Whenever there is a difference in elevation between adjacent travel lanes, the W8-11 sign with "UNEVEN LANES" is required at intervals of ⅓ mile maximum.
- 3. If D is $1\frac{1}{2}$ " or less, no treatment is required.
- 4. Treatment allowed only when D is 3" or less.
- 5. If the slope is steeper than 1:4 (not to be steeper than 1:1), the R4-1 and MOT-1-06 signs shall be used as a supplement to the W8-11; this condition should never exceed 3 miles in length.



TRAVEL LANE TREATMENT FOR MILLING OR RESURFACING DETAIL

PEDESTRIAN WAY DROP-OFF CONDITION NOTES

- 1. A pedestrian way drop-off is defined as:
- a. a drop in elevation greater than 10" that is closer than 2' from the edge of the pedestrian way
- b. a slope steeper than 1:2 that begins closer than 2' from the edge of the pedestrian way when the total drop-off is greater than 60"
- 2. Protect any drop-off adjacent to a pedestrian way with pedestrian longitudinal channelizing devices, temporary barrier wall, or approved handrail.

DROP-OFFS IN WORK ZONES

REVISION 11/01/18

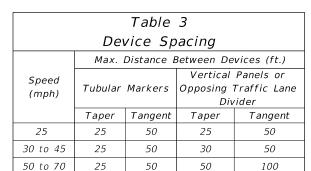
DESCRIPTION:

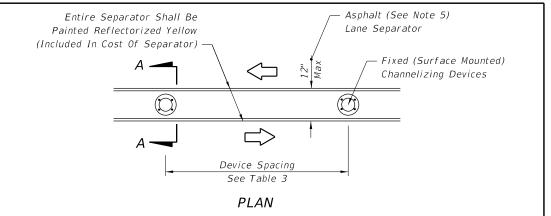




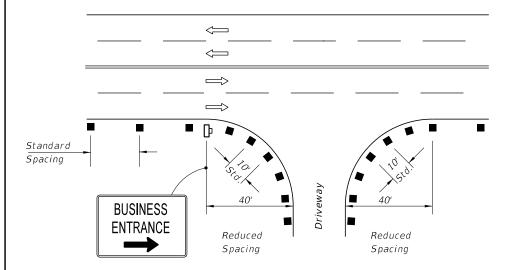








B/0



1. For single business entrances, place one 24" x 36" business sign for each

Index 700-102 may be used when approved by the Engineer.

which is often the case with resurfacing type projects.

common driveway entrance.

driveway entrance affected. Signs shall show specific business names. Logos

2. When several businesses share a common driveway entrance, place one 24" x 36"

standard BUSINESS ENTRANCE sign in accordance with Index 700-102 at the

3. Channelizing devices shall be placed at a reduced spacing on each side of the driveway entrance, but shall not restrict sight distance for the driveway users.

4. Business entrance signs are intended to guide motorist to business entrances moved/modified or disturbed during construction projects. Business entrance

signs are not required where there is minimal disruption to business driveways

PLACEMENT OF BUSINESS ENTRANCE SIGNS AND

CHANNELIZING DEVICES AT BUSINESS ENTRANCE

may be provided by business owners. Standard BUSINESS ENTRANCE sign in

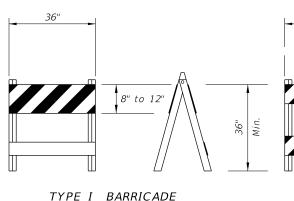
- Two 3" White Retroreflectorized Bands -2"-6" Space Asphalt (See Note 5) 12" Lane Separator 12" Max Tubular Marker Vertical Panel Opposing Traffic Orange O/W Lane Divider W6-4
 - FIXED (SURFACE MOUNTED) CHANNELIZING DEVICES

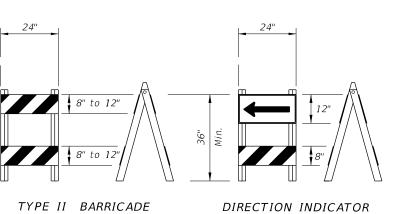
SECTION AA

- 1. Temporary lane separators shall be supplemented with any of the following approved fixed (surface mounted) channelizing devices: tubular markers, vertical panels, or opposing traffic lane divider panels. Opposing traffic lane divider panels (W6-4) shall only be used as center lane dividers to separate opposing vehicular traffic on a two-lane, two-way operation. Tubular Markers, Vertical Panels and Opposing Traffic Lane Divider panels shall not be intermixed within the limits where the temporary lane separator is used. The connection between the channelizing device and the temporary lane separator curb shall hold the channelizing device in a vertical position.
- 2. Reflectorized materials shall have a smooth sealed outer surface which will display the same approximate color day and night. Furnish channelizing devices having retroreflective sheeting meeting the requirements of Section 990.
- 3. 12" openings for drainage shall be constructed in the asphalt and portable temporary lane separator at a maximum spacing of 25' in areas with grades of 1% or less or 50' in areas with grades over 1% as directed by the Engineer.
- 4. Tapered ends shall be used at the beginning and end of each run of the temporary lane separator to form a gradual increase in height from the pavement level to the top of the temporary lane separator.
- 5. The Contractor has the option of using portable temporary lane separators containing fixed channelizing devices in lieu of the temporary asphalt separator and channelizing devices detailed on this sheet. The portable temporary lane separator shall come in portable sections that can be connected to maintain continuous alignment between the separate curb sections. Each temporary lane separator section shall be 36 inches to 48 inches in total length. Portable temporary lane separators shall duplicate the color of the pavement marking. Portable temporary lane separators shall be one of those listed on the Approved Products List.
- 6. Any damage to existing pavement caused by the removal of temporary lane separator shall be satisfactorily repaired and the cost of such repairs are to be included in the cost of Maintenance of Traffic, LS.

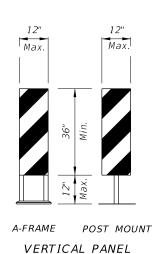
TEMPORARY LANE SEPARATOR

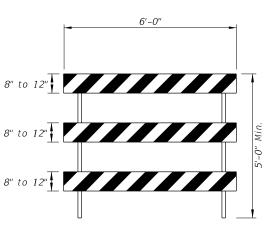
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**BARRICADE** 





TYPE III BARRICADE

## *=CHANNELIZING DEVICES*=

### CHANNELIZING DEVICE NOTES:

TUBULAR NON-FIXED MARKER TO BE USED DURING DAYLIGHT ONLY

- 1. The details shown on this sheet are for the following purposes:
- a. For ease of identification and
- b. To provide information that supplements or supersedes that provided by the MUTCD.
- 2. The Type III Barricade shall have a unit length of 6'-0" only. When barricades of greater lengths are required those lengths shall be in multiples of the 6'-0" unit.
- 3. No sign panel should be mounted on any channelizing device unless the channelizing device/sign combination was found to be crashworthy and the sign panel is mounted in accordance with the vendor drawing for the channelizing device shown on the Approved Products List (APL).
- 4. Ballast shall not be placed on top rails or any striped rails or higher than 13" above the driving surface.
- 5. The direction indicator barricade may be used in tapers and transitions where specific directional guidance to drivers is necessary. If used, direction indicator barricades shall be used in series to direct the driver through the transition and into the intended travel lane.
- 6. The splicing of sheeting is not permitted on either channelizing devices or MOT signs.
- 7. For rails less than 3'-0" long, 4" stripes shall be used.
- 8. Cones shall:

DESCRIPTION:

- a. Be used only in active work zones where workers are present.
- b. Be reflectorized as per the MUTCD with Department-approved reflective collars when used at night.
- 9. Vehicular longitudinal channelizing devices shall not exceed 36" in height. For vehicular longitudinal channelizing devices (LCDs) less than 32" in height, the LCD shall be supplemented with approved fixed (surface mounted) channelizing devices (tubular markers, vertical panels, etc.) along the run of the LCD, at the ends, at 50' centers on tangents, and 25' centers on radii. The cost of the fixed supplemented channelizing devices shall be included in the cost of the LCD. LCDs less than 32" in height shall not be used for speeds greater than 45 mph.

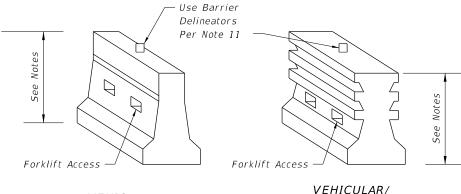
- 10. For pedestrian longitudinal channelizing devices, the device shall have a minimum of 8" continuous detectable edging above the walkway. A gap not exceeding a height of 2" is allowed to facilitate drainage. The top surface of the device shall be a minimum height of 32" and have a ½" or less difference in any plane at all connection points between the devices to facilitate hand trailing. The bottom and the top surface of the device shall be in the same vertical plane. If pedestrian dropoff protection is required, the device shall have a footprint or offset of at least 2', otherwise the device must be at least 42" in height above the walkway and be anchored or ballasted to withstand a 200 lb lateral point load at the top of the device.
- 11. For Barrier Delineators, see Specification 102. Place on top of unit so that retroreflective sheeting faces vehicular traffic. Color must match adjacent longitudinal pavement marking.

### TEMPORARY BARRIER NOTES:

1. Where a barrier is specified, any of the types below may be used in accordance with the applicable Index:

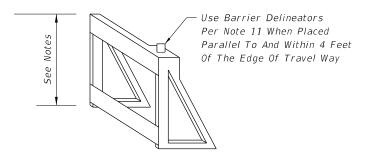
IndexDescription102-100Temporary Barrier102-120Low Profile Barrier536-001Guardrail

2. Trailer Mounted Barriers may be used to provide positive protection for workers within the work areas. APL drawings may be used as a guide to develop project specific Temporary Traffic Control Plans that are signed and sealed by the Contractor's Engineer.



VEHICULAR LCD

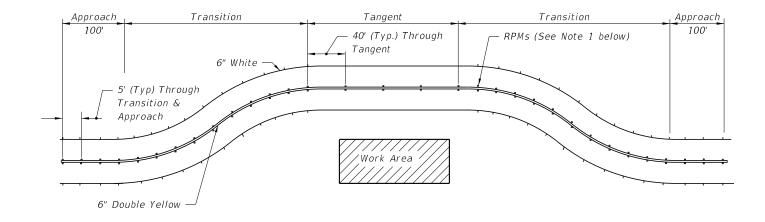
PEDESTRIAN LCD



PEDESTRIAN LCD

LONGITUDINAL CHANNELIZING DEVICE

LAST REVISION 11/01/17

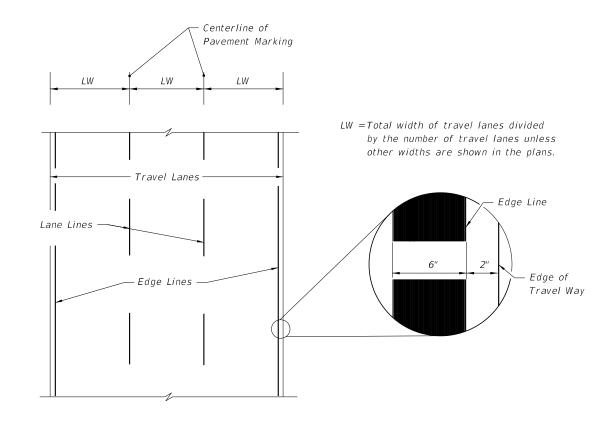


### USE OF RPMS TO SUPPLEMENT PAINT OR REMOVABLE TAPE IN WORK ZONES

- 1. RPMs shall be installed as a supplement to:
- a. All lane lines.
- b. Edge lines in transition & approach areas.
- c. Edge lines of gore areas.
- 2. Placement of RPMs should be as shown in Index 706-001 with the following exceptions: RPMs shall be placed at 5 feet center to center in approach and transition areas.

### NOTES FOR RAISED PAVEMENT MARKERS:

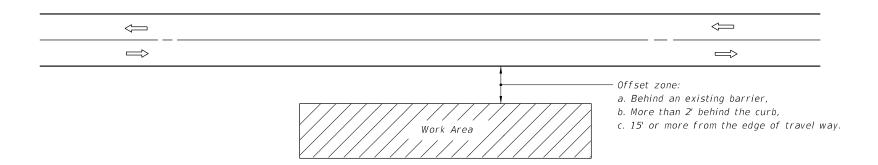
- 1. The color of the raised pavement marker under both day and night conditions shall conform to the color of the marking for which they serve as a positioning guide, or for which they supplement.
- 2. RPMs used to supplement lane lines are to be paid for as Raised Pavement Marker (Temporary), EA. RPMs used as a temporary substitute for paint or removable tape due to equipment malfunction are to be placed at the Contractor's expense.



PLACEMENT OF PAVEMENT MARKINGS

PAVEMENT MARKINGS

DESCRIPTION:



- 1. If the work operation (excluding establishing and terminating the work area) requires that two or more work vehicles cross the offset zone in any one hour, traffic control will be in conformance with Index 102-602.
- 2. No special signing is required.
- 3. When a side road intersects the highway within the work area, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 4. When construction activities encroach on a sidewalk refer to Index 102-660.
- 5. For general TCZ requirements and additional information, refer to Index 102-600.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS AND THEIR ACTIVITIES ARE BEHIND AN EXISTING BARRIER, MORE THAN 2' BEHIND THE CURB, OR 15' OR MORE FROM THE EDGE OF TRAVEL WAY.

#### SYMBOLS



Work Area

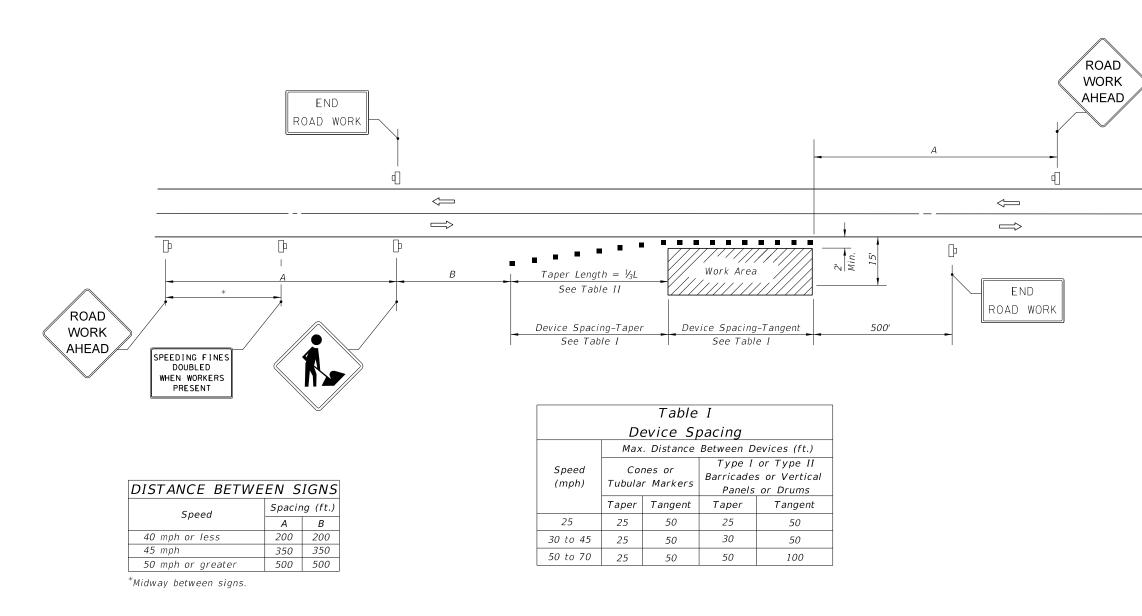
≥ DESCRIPTION:



Lane Identification + Direction of Traffic

LAST REVISION 11/01/17

FDOT



#### SYMBOLS

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

Lane Identification + Direction of Traffic

#### GENERAL NOTES

- 1. When four or more work vehicles enter the through traffic lanes in a one hour period or less (excluding establishing and terminating the work area), the advanced FLAGGER sign shall be substituted for the WORKERS sign. For location of flaggers and FLAGGER signs, see Index 102-603.
- 2. SHOULDER WORK sign may be used as an alternate to the WORKER symbol sign only on the side where the shoulder work is being performed.
- 3. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 4. For general TCZ requirements and additional information, refer to Index 102-600.

#### DURATION NOTES

- 1. Signs and channelizing devices may be omitted if all of the following conditions are met:
- a. Work operations are 60 minutes or less.
- b. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.

	-			
Tap	er Le	ngth -	- Shot	ulder
Enood		⅓L (ft)		
Speed (mnh)	0'	10'	121	Notes

Table II

	Speed		⅓L (ft)		N/-4
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	(mph)	8'	10'	12'	Notes
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		Shldr.	Shldr.	Shldr.	
35     55     68     82     60       40     72     90     107       45     120     150     180       50     133     167     200	25	28	35	42	
35     55     68     82     60       40     72     90     107       45     120     150     180       50     133     167     200	30	40	50	60	$L = \frac{WS^2}{}$
45     120     150     180       50     133     167     200	35	55	68	82	60
50 133 167 200	40	72	90	107	
31 111 111	45	120	150	180	
55 147 183 220	50	133	167	200	
1 1 1 1	55	147	183	220	
60 160 200 240 L=WS	60	160	200	240	L=WS
65 173 217 260	65	173	217	260	
70 187 233 280	70	187	233	280	

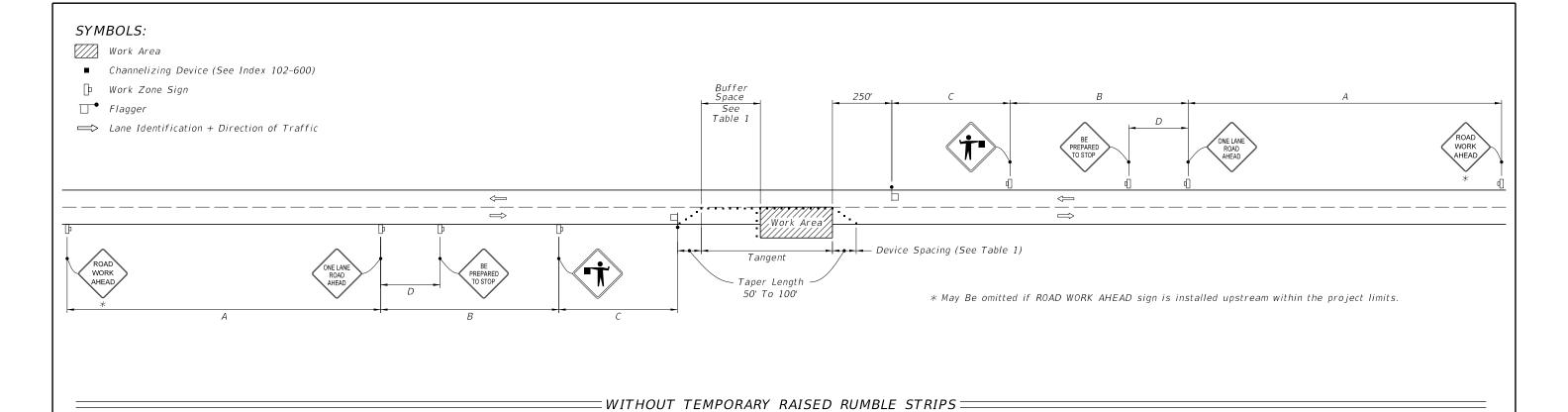
- minimum shoulder width
- $V_3L$  = Length of shoulder taper in feet
- W = Width of total shoulder in feet(combined paved and unpaved width)
- S = Posted speed limit (mph)

### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF TRAVEL WAY.

**REVISION** 11/01/17

FDOT



- 1. Special Conditions may be required in accordance with these notes and the following sheets:
- A. Railroad Crossings:
- a. If an active railroad crossing is located closer to the Work Area than the queue length plus 300 feet, extend the Buffer Space as shown on Sheet 3.
- b. If the queuing of vehicles across an active railroad crossing cannot be avoided, provide a uniformed traffic control officer or flagger at the highway-rail grade crossing to prevent vehicles from stopping within the highway-rail grade crossing, even if automatic train warning devices are in place.
- B. If the Work Area encroaches on the Centerline, use the Layout for Temporary Lane Shift to Shoulder on Sheet 3 only if the Existing Paved Shoulder width is sufficient to provide for an 11' lane between the Work Area and the Edge of Existing Paved Shoulder. Reduce the posted speed when appropriate.
- 2. Temporary Raised Rumble Strips:
- A. Use when both of the following conditions are met concurrently: a. Existing Posted Speed is 55 mph or greater;
- b. Work duration is greater than 60 minutes.
- B. Use a consistent Strip color throughout the work zone.
- C. Place each Rumble Strip Set transversely across the lane at locations shown.
- D. Use Option 1 or Option 2 as shown on Sheet 2. Use only one option throughout work zone.
- 3. Additional one-way control may be provided by the following means:
- A. Flag-carrying vehicle;

DESCRIPTION:

- B. Official vehicle;
- C. Pilot vehicles;
- D. Traffic signals.

When flaggers are the sole means of one-way control, the flaggers must be in sight of each other or in direct communication at all times.

- 4. When a side road intersects the highway within the TTC zone, place additional TTC devices in accordance with other applicable TCZ Indexes.
- 5. The two channelizing devices directly in front of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 6. When Buffer Space cannot be attained due to geometric constraints, use the greatest attainable length, not less than 200 ft, for posted speeds greater than 25 mph.
- 7. ROAD WORK AHEAD and the BE PREPARED TO STOP signs may be omitted if all of the following conditions are met:
  - A. Work operations are 60 minutes or less.
  - B. Speed limit is 45 mph or less.
  - C. There are no sight obstructions to vehicles approaching the work area for a distance equal to the Buffer Space shown in Table 1.
  - D. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
  - E. Volume and complexity of the roadway has been considered.
  - F. If a railroad crossing is present, vehicles will not queue across rail tracks.
  - G. AFADs are not in use.
- 8. See Index 102-600 for general TCZ requirements and additional information.
- 9. Automated Flagger Assistance Devices (AFADs) may be used in accordance with Specifications Section 102, 990 and the APL vendor drawings.

	TABLE 1								
	DEVICE SPACING								
Posted Speed	of Co	n Spacing nes or Markers	Maximum Spacing of Type I or Type II Barricades/Panels/Drums		Distance Between Signs			Buffer Space	
	On a On a On a Taper Tangent Taper Tangent		A	В	С	D			
25	20'	50'	20'	50'	200'	200'	200'	100'	155'
30	20'	50'	20'	50'	200'	200'	200'	100'	200'
35	20'	50'	20'	50'	200'	200'	200'	100'	250'
40	20'	50'	20'	50'	200'	200'	200'	100'	305'
45	20'	50'	20'	50'	350'	350'	350'	175'	360'
50	20'	50'	20'	100'	500'	500'	500'	250'	425'
55	20'	50'	20'	100'	2640'	1500'	1000'	500'	495'
60	20'	50'	20'	100'	2640'	1500'	1000'	500'	570'
65	20'	50'	20'	100'	2640'	1500'	1000'	500'	645'
70	20'	50'	20'	100'	2640'	1500'	1000'	500'	730'

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF TRAVEL WAY.

LAST REVISION 11/01/17

FDOT

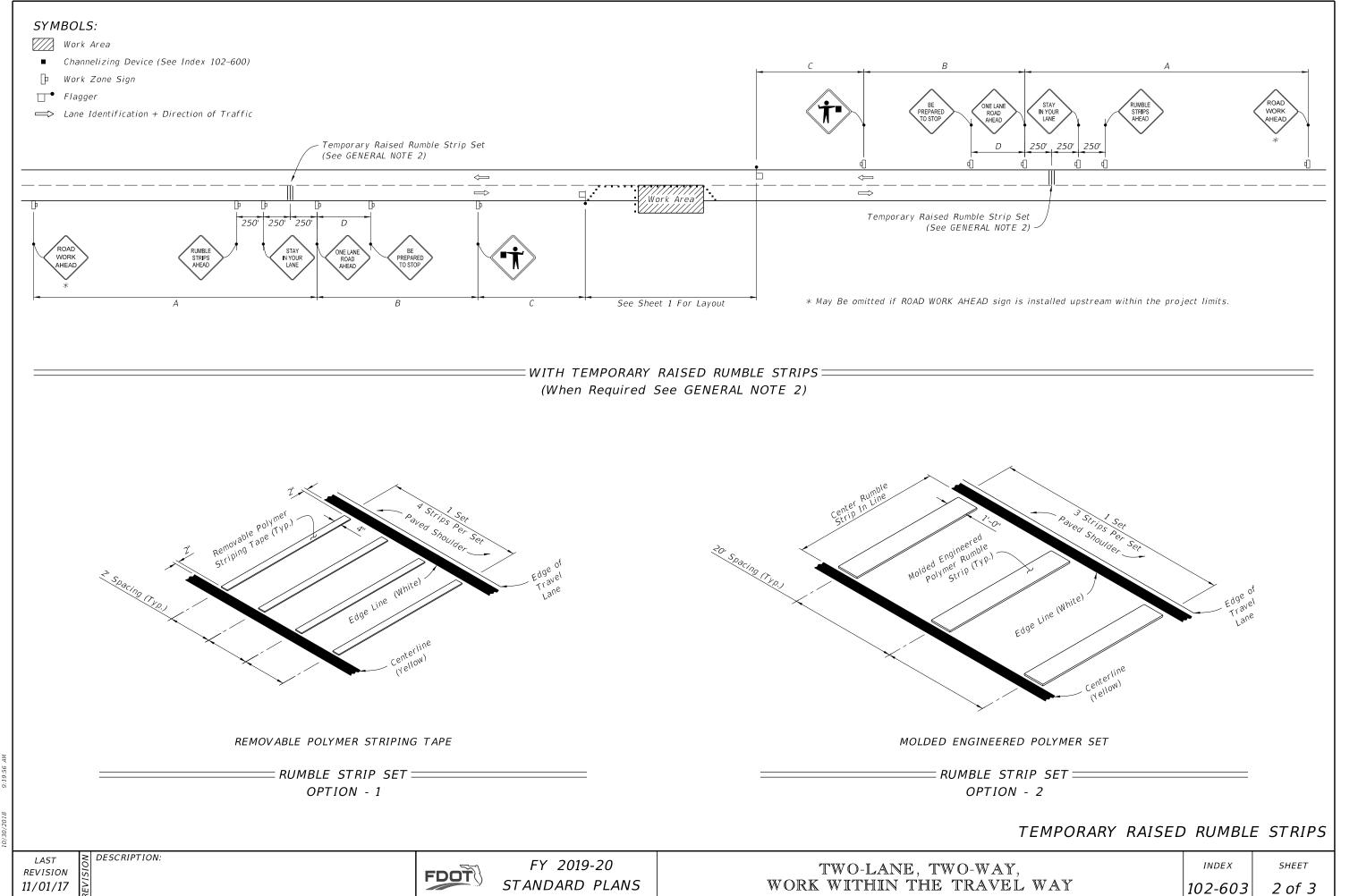
FY 2019-20
STANDARD PLANS

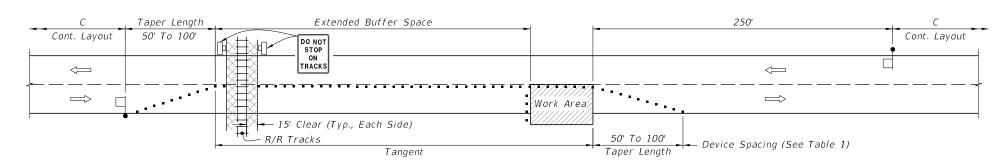
TWO-LANE, TWO-WAY, WORK WITHIN THE TRAVEL WAY

INDEX

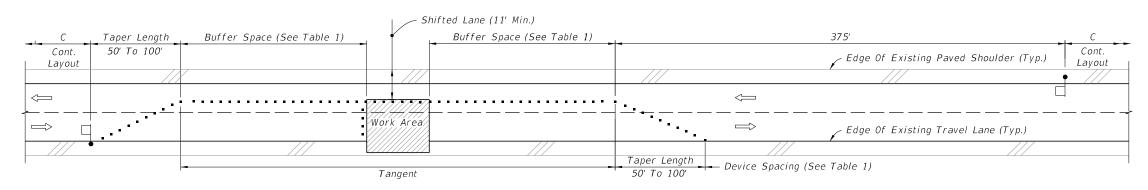
SHEET

102-603 1 of 3





#### TEMPORARY RAILROAD CROSSING BUFFER SPACE EXTENSION



TEMPORARY LANE SHIFT TO SHOULDER WHEN WORK AREA ENCROACHES ON THE CENTERLINE

SPECIAL CONDITIONS

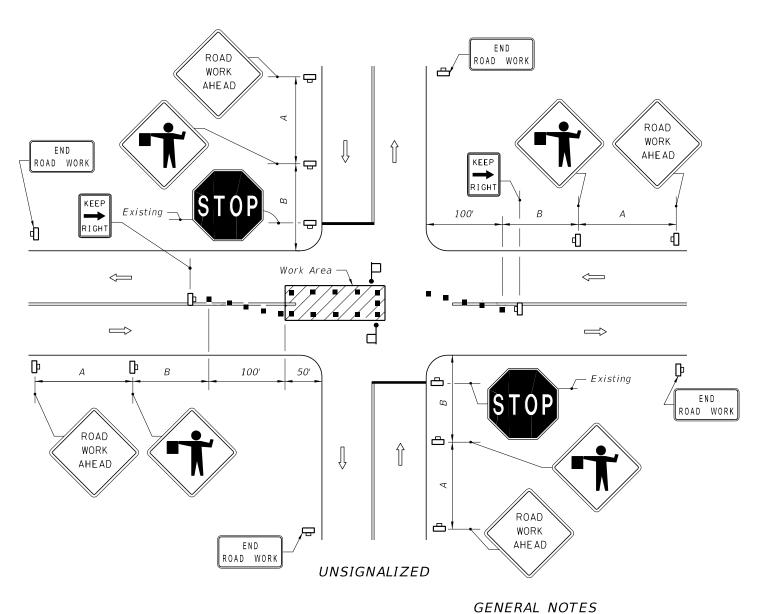
Cross Reference:

1. See General Note 1, Sheet 1 for more information.

SPECIAL CONDITIONS

LAST REVISION 11/01/17

≥ DESCRIPTION:



Work Area

Channelizing Device (See Index 102-600)

SYMBOLS

Work Zone Sign

Flagger

Stop Bar

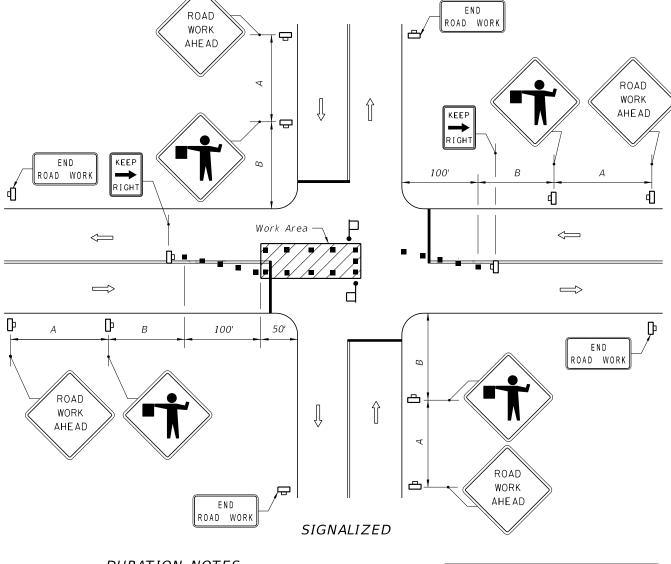
Lane Identification + Direction of Traffic

DESCRIPTION:

- 1. The FLAGGER legend sign may be substituted for the symbol sign.
- 2. When vehicles in a parking zone block the line of sight to TCZ signs, the signs shall be post mounted and located in accordance with Index 700-101.
- 3. If the work space extends across a crosswalk, the crosswalk should be closed using the information in Index 102-660.
- 4. Flaggers shall be located where they can control more than one direction of

Flaggers shall be in sight of each other or in direct communication at all times.

- 5. Maximum spacing between channelizing devices shall be not greater than 20'.
- 6. Temporary signal phasing modifications are to be approved by the District Traffic Operations Engineer prior to the beginning of work.
- 7. For general TCZ requirements and additional information, refer to Index 102-600.
- 8. For unsignalized intersections, use Temporary Raised Rumble Strips in accordance with Index 102-603. Placement of Rumble Strips and additional signs should begin at FLAGGER sign location.



#### **DURATION NOTES**

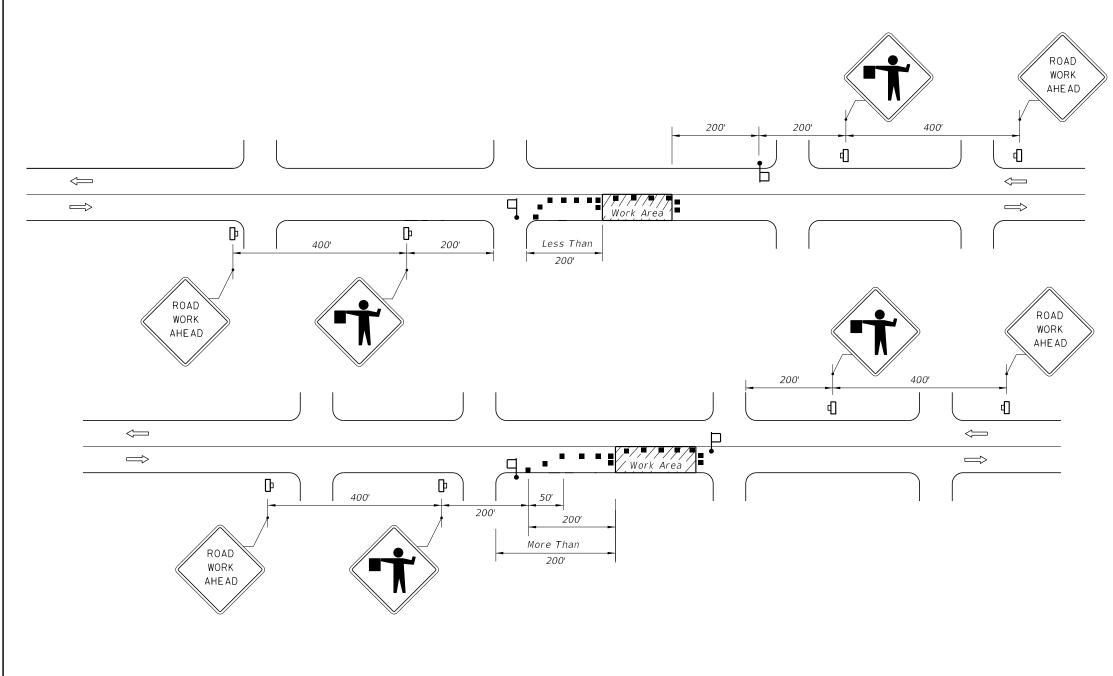
- 1. ROAD WORK AHEAD AND END ROAD WORK sign may be omitted if all of the following conditions are met:
  - a. Work operations are 60 minutes or less.
  - b. Speed is 45 mph or less.
  - c. No sight obstructions to vehicles approaching the work area for a distance equal to A plus B.
  - d. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- e. Volume and complexity of the roadway has been considered.

DISTANCE BETWEEN SIGNS			
Speed	Spacing (ft.)		
Speed	Α	В	
40 mph or less	200	200	
45 mph	350	350	

#### **CONDITIONS**

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF A PORTION OF ONE OR MORE TRAFFIC LANES IN AN INTERSECTION.

**REVISION** 11/01/17



#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF ONE TRAFFIC LANE, FOR WORK AREAS LESS THAN 200' DOWNSTREAM FROM AN INTERSECTION FOR A PERIOD OF MORE THAN 60 MINUTES.

#### **CONDITIONS**

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF ONE TRAFFIC LANE, FOR WORK AREAS 200' OR MORE DOWNSTREAM FROM AN INTERSECTION FOR A PERIOD OF MORE THAN 60 MINUTES.

## **DURATION NOTES**

**SYMBOLS** Work Area

- Channelizing Device (See Index 102-600)
- Work Zone Sign
- Flagger
- Lane Identification + Direction of Traffic

DESCRIPTION:

### GENERAL NOTES

- 1. Work operations shall be confined to one travel lane, leaving the opposing travel lane open to traffic.
- 2. When vehicles in a parking zone block the line of sight to TCZ signs or when TCZ signs encroach on a normal pedestrian walkway, the signs shall be post mounted and located in accordance with Index 700-101.
- 3. If work area is confined to an outside auxiliary lane, the work area shall be barricaded and the FLAGGER signs replaced by ROAD WORK AHEAD signs. Flaggers are not required.
- 4. Flaggers shall be in sight of each other or in direct communication at all times.

- 5. The FLAGGER legend sign may be substituted for the symbol sign.
- 6. The maximum spacing between devices shall be no greater than 25.
- 7. For general TCZ requirements and additional information, refer to Index 102-600.
- 8. The two channelizing devices directly in front and directly at the end of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 9. Use Temporary Raised Rumble Strips in accordance with Index 102-603. Placement of Rumble Strips and additional signs should begin at FLAGGER sign location.

- 1. ROAD WORK AHEAD sign may be omitted if all of the following conditions are met:
- a. Work operations are 60 minutes or less.
- b. Speed is 45 mph or less.
- c. No sight obstructions to vehicles approaching the work area for a distance of 600 feet.
- d. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- e. Volume and complexity of the roadway has been considered.

**REVISION** 11/01/17

FDOT

FY 2019-20 STANDARD PLANS

TWO-LANE, TWO-WAY, WORK NEAR INTERSECTION

INDEX

SHEET

102-605 1 of 1

- 1. Use either portable signals or span wire signals and include two signal faces for each approach.
- 2. Obtain approval from the District Traffic Operations Engineer for the installation and timing of the signals prior to the signals being placed in operation. Adjust timing based on changing field conditions as approved by the Worksite Traffic Supervisor. Obtain approval from the District Traffic Operations Engineer for any timing changes that are either reoccurring or last longer than 24 hours.
- 3. For the maximum distance between portable distance between portable temporary traffic signals do not exceed the distance at which the signals can safely communicate. When the distance between signals is 0.25 miles to 0.50 miles, use a countdown timer on both signals. When the distance between signals is greater than 0.50 miles, use a combination of a pilot vehicle and manually controlled temporary traffic signals.
- 4. The SIGNAL AHEAD legend sign may be substituted for the symbol sign.
- 5. Use Type III Barricades to block haul road access when the haul road is not in operation and a flagger/signal operator is not on duty, except when the haul road is an existing properly marked road.
- 6. Monitor temporary traffic signals by having one or more workers present during operation. In the event of a temporary traffic signal failure, maintain traffic with flaggers.
- 7. Use Temporary Raised Rumble Strips in accordance with Index 102-603.

#### SYMBOLS

Work Area

Work Zone Sign

Temporary Traffic Signal

Channelizing Device (See Index 102-600)

Type III Barricade

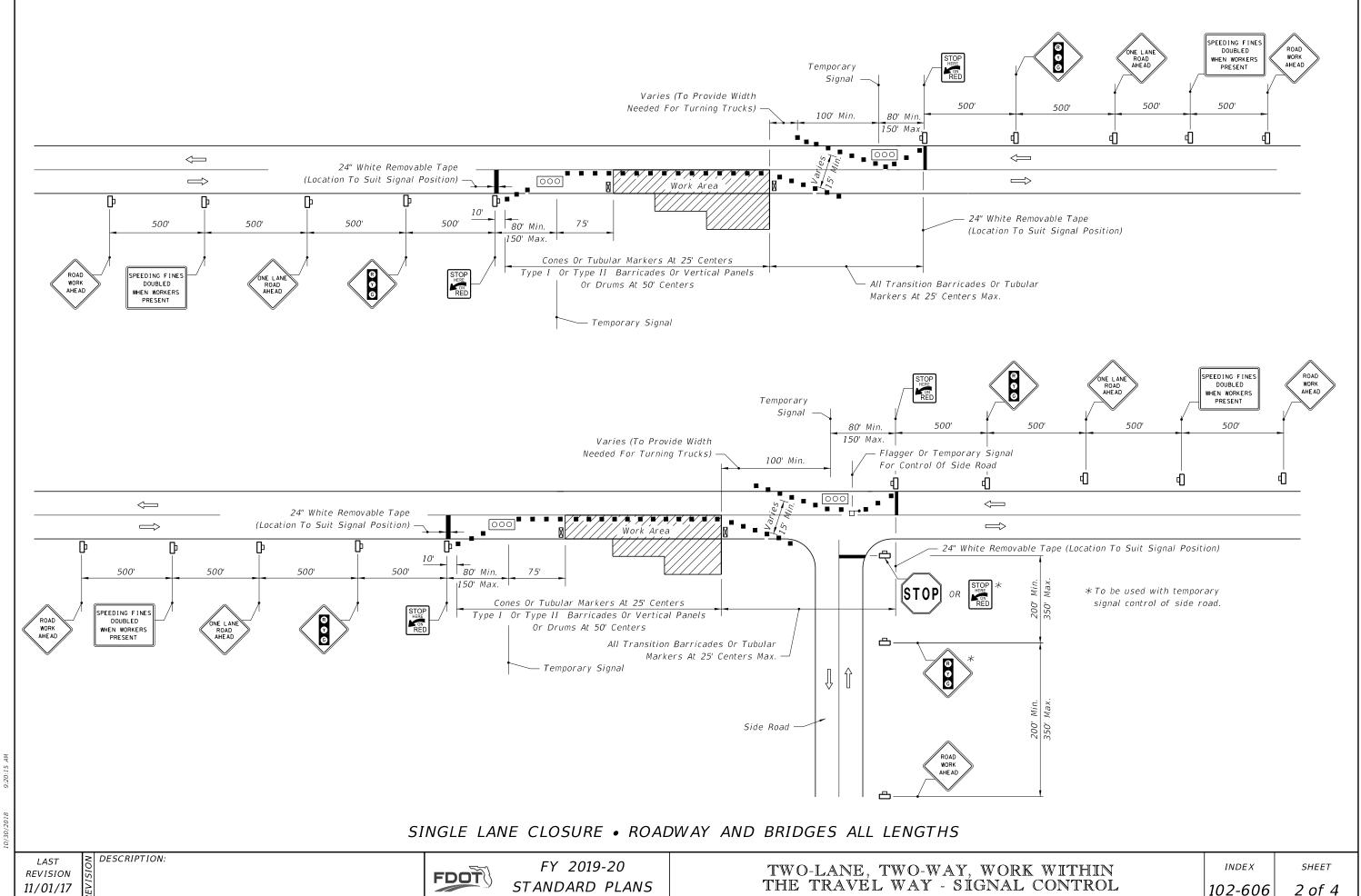
Stop Bar

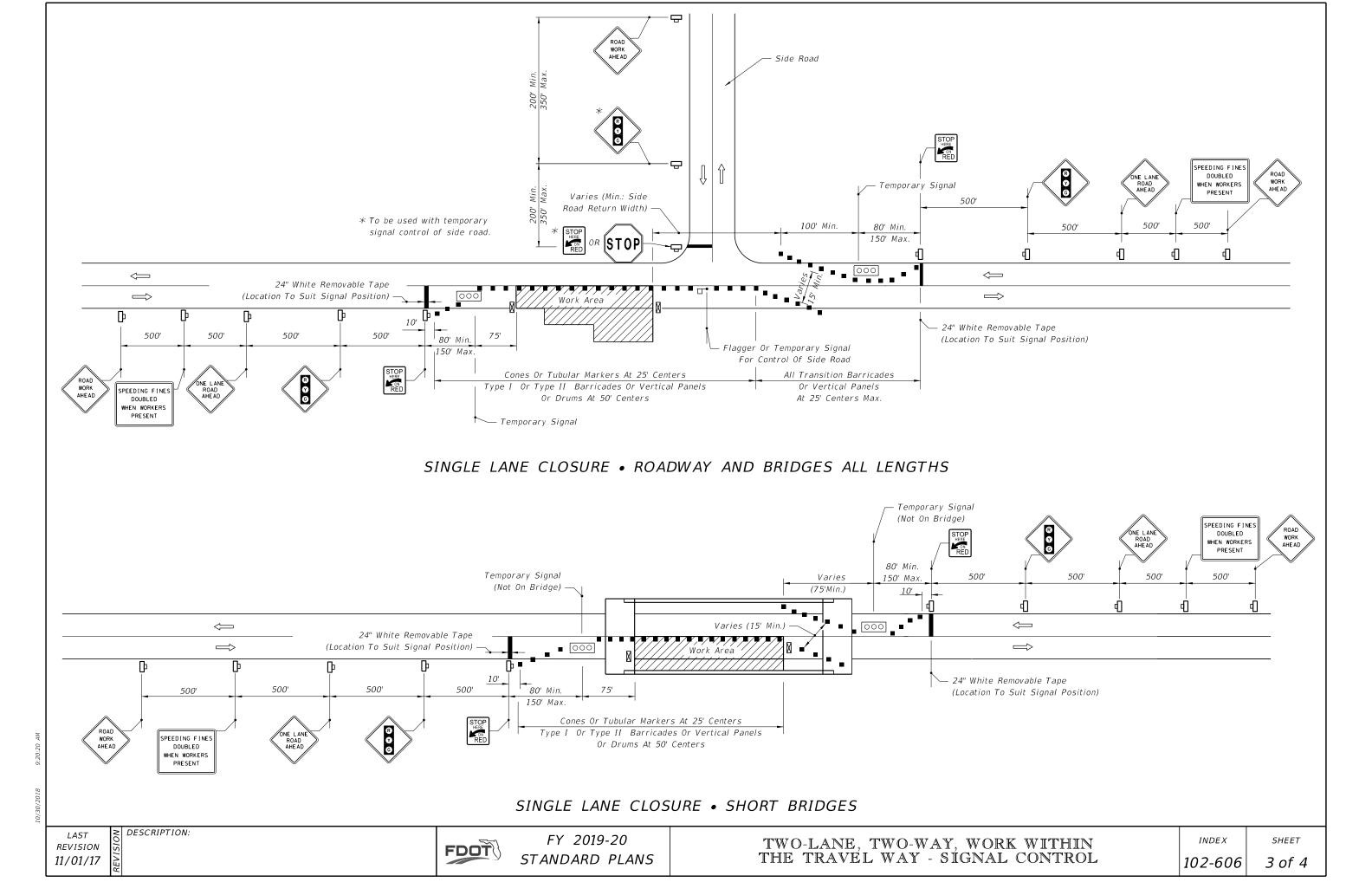
Flagger

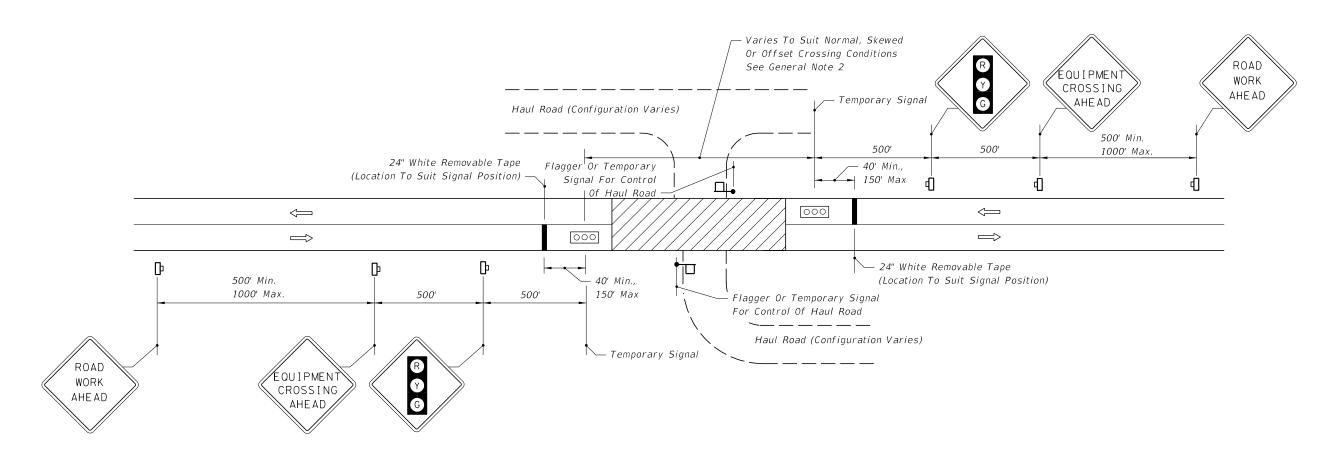
Lane Identification + Direction of Traffic

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES WILL ENCROACH ON ONE LANE OR MOMENTARILY ENCROACH ON BOTH LANES OF A TWO-LANE TWO-WAY ROADWAY AND TRAFFIC SIGNALS ARE NEEDED.







MOMENTARY ROADWAY CLOSURE . HAUL ROUTE CROSSING

REVISION 11/01/17

DESCRIPTION:

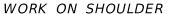
FDOT

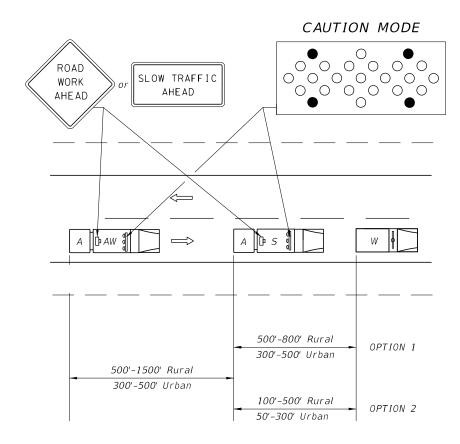
FY 2019-20 STANDARD PLANS

THE TRAVÉL WAY - SÍGNAL CONTROL

INDEX

SHEET





OPTION 1: Advanced Warning Vehicle is optional and to be operated on the shoulder when feasible. If an Advance Warning Vehicle is operated in the shoulder, an approved Truck Mounted Attenuator is required on both the Advance Warning and Shadow Vehicles. If an Advance Warning Vehicle is operated in the lane behind the Shadow Vehicle, an approved Truck Mounted Attenuator will be required on the Advance Warning Vehicle, but not required on the Shadow Vehicle. The Advance Waning Arrow Board and Warning Sign is required on both the Advance warning and Shadow Vehicles.

OPTION 2: Advanced Warning Vehicle is required and must be operated in the lane behind the shadow vehicle. An approved Truck Mounted Attenuator will be required on the Advanced Warning Vehicle but not required on the Shadow Vehicle. The Advance Warning Arrow Board and Warning Sign is required on both the Advance Warning and Shadow Vehicles.

> WORK IN TRAVEL WAY (Option 2 Shown, Option 1 Similar)

#### SYMBOLS

Work Area



Work Zone Sign

DESCRIPTION:



Lane Identification + Direction of Traffic Work Vehicle With Rotating/Strobe Lights



Shadow (S) Or Advance Warning (AW) Vehicle with Advance Warning Arrow



Truck/Trailer Mounted Attenuator (TMA)



Advanced Warning Arrow Board

Board and Sign Message

#### GENERAL NOTES

- 1. Where work activities within 2' of the edge of travel way are incidental (i.e., Mowing, Litter Removal), the Engineer may delete requirements for signs and the advance warning vehicle provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 2. If an arrow board is used, the caution mode shall be used.
- 3. Shadow and Advance Warning Vehicle shall display rotating/strobe lights.
- 4. For general TCZ requirements and additional information, refer to Index 102-600.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES REQUIRE AN INTERMITTENT OR CONTINUOUS MOVING OPERATION.

**REVISION** 11/01/17



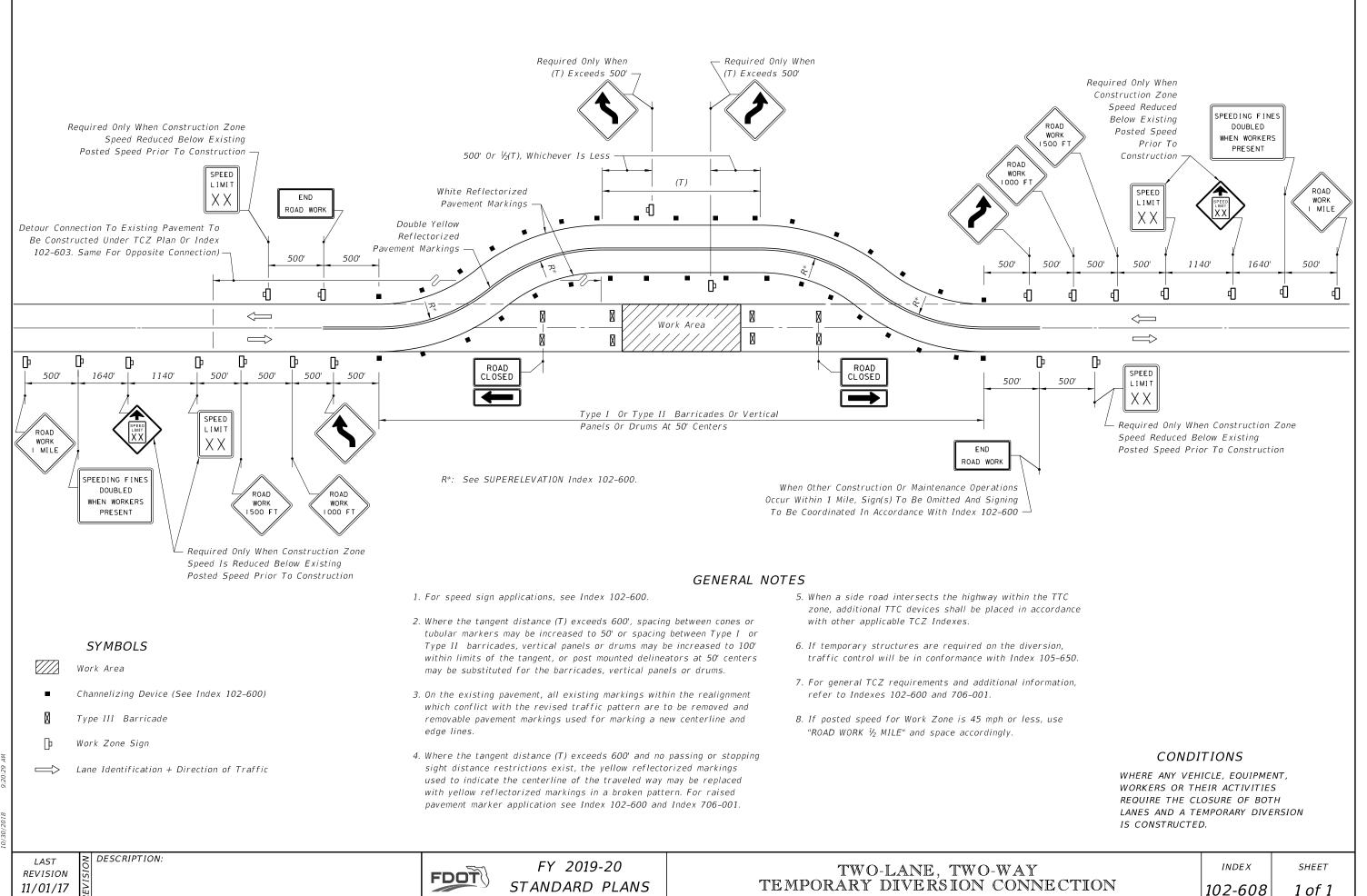
FY 2019-20 STANDARD PLANS

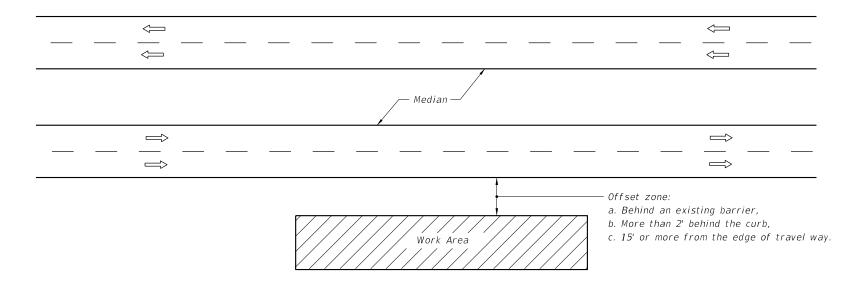
TWO-LANE, TWO-WAY MOBILE OPERATION, WORK

INDEX

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102-607





- 1. If the work operation (excluding establishing and terminating the work area), requires that two or more work vehicles cross the offset zone in any one hour, traffic control will be in accordance with Index 102-612.
- 2. No special signing is required.
- 3. This index also applies when work is being performed on a multilane undivided highway.
- 4. This index also applies to work performed in the median behind an existing barrier or more than 15' from the edge of travel way, both roadways. Work performed in the median behind curb and gutter shall be in accordance with Index 102-612.
- 5. When a side road intersects the highway within the work area, additional traffic control devices shall be placed in accordance with other applicable TCZ Indexes.
- 6. When construction activities encroach on a sidewalk, refer to Index 102-660.
- 7. For general TCZ requirements and additional information, refer to Index 102-600.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS AND THEIR ACTIVITIES ARE BEHIND AN EXISTING BARRIER, MORE THAN 2' BEHIND THE CURB, OR 15' OR MORE FROM THE EDGE OF TRAVEL WAY.

**REVISION** 11/01/17

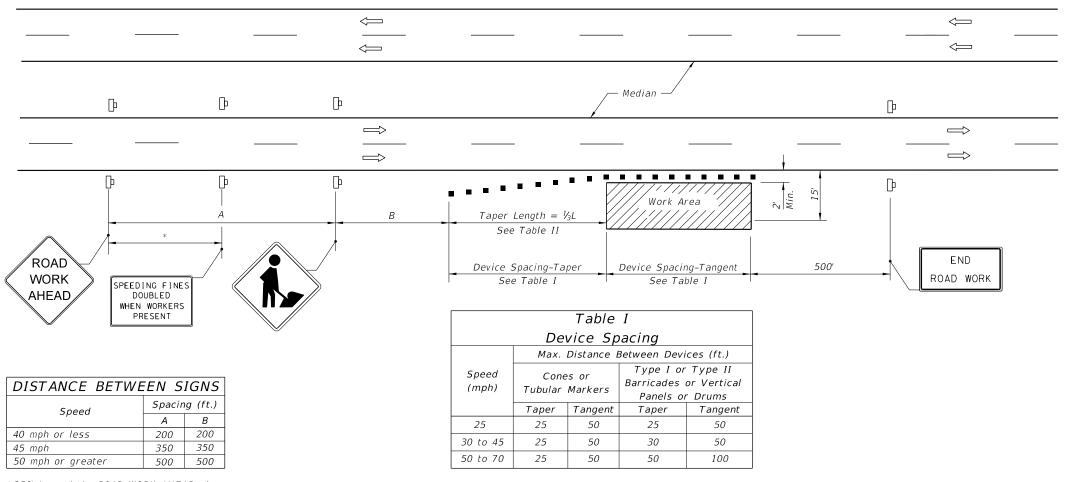
DESCRIPTION:

**SYMBOLS** 

Lane Identification + Direction of Traffic

Work Area





*250' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

#### GENERAL NOTES

- 1. When a high volume of work vehicles are entering and leaving the Work Area at speeds slower than 10 MPH below the posted speed, place an MOT-5-06 sign in the ROAD WORK AHEAD sign location and shift the ROAD WORK AHEAD sign upstream 500 ft.
- 2. This TCZ plan also applies to work performed in the median more than 2' but less than 15' from the edge of travelway.
- 3. When work is being performed on a multilane undivided roadway the signs normally mounted in the median (as shown) shall be omitted.
- 4. WORKERS signs to be removed or fully covered when no work is being performed.
- 5. SHOULDER WORK sign may be used as an alternate to the WORKER symbol sign.
- 6. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 7. For general TCZ requirements and additional information, refer to Index 102-600.

- 1. Signs and channelizing devices may be omitted if all of the
- b. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.

# Table II Taper Length - Shoulder

Speed		⅓L (ft.)		Natas
(mph)	8'	10'	12'	Notes
	Shldr.	Shldr.	Shldr.	
25	28	35	42	
30	40	50	60	$I = \frac{WS^2}{I}$
35	55	68	82	60
40	72	90	107	
45	120	150	180	
50	133	167	200	
55	147	183	220	, ,,,
60	160	200	240	L=WS
65	173	217	260	
70	187	233	280	

8' minimum shoulder width.

 $\frac{1}{3}L$  = Length of shoulder taper in feet

W = Width of total shoulder in feet(combined paved and unpaved width)

S = Posted speed limit (mph)

### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF TRAVEL WAY.

#### **SYMBOLS**

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

Lane Identification + Direction of Traffic

#### **DURATION NOTES**

- following conditions are met:
- a. Work operations are 60 minutes or less.

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FDOT

DISTANCE BETWEEN SIGNS			
Speed	Spacing (ft.)		
Speed	Α	В	С
40 mph or less	200	200	200
45 mph	350	350	350
50 mph	500	500	500
*55 mph or greater	2640	1640	1000

WHEN WORKERS
PRESENT

- * The ROAD WORK 1 MILE sign may be used as an alternate to the ROAD WORK AHEAD sign and the RIGHT LANE CLOSED ½ MILE sign may be used as an alternate to the RIGHT LANE CLOSED AHEAD sign.
- ** 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

#### SYMBOLS

//// Work Area

■ Channelizing Device (See Index 102-600)

₩ork Zone Sign

••• Advance Warning Arrow Board

DESCRIPTION:

#### GENERAL NOTES

- 1. Work operations shall be confined to one traffic lane, leaving the adjacent lane open to traffic.
- 2. On undivided highways the median signs as shown are to be omitted.
- 3. When work is performed in the median lane on divided highways, the channelizing device plan is inverted and left lane closed and lane ends signs substituted for the right lane closed and lane end signs.

The same applies to undivided highways with the following exceptions:

- a. Work shall be confined within one median lane.
- b. Additional barricades, cones, or drums shall be placed along the centerline abutting the work area and across the trailing end of the work area.

When work on undivided highways occurs across the centerline so as to encroach on both median lanes, the inverted plan is applied to the approach of both roadways.

- 4. Signs and traffic control devices are to be modified in accordance with INTERMITTENT WORK STOPPAGE details (sheet 2 of 2) when no work is being performed and the highway is open to traffic.
- 5. The two channelizing devices directly in front of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 6. When paved shoulders having a width of 8 ft. or more are closed, channelizing devices shall be used to close the shoulder in advance of the merging taper to direct vehicular traffic to remain within the travel way. See Index 102-612 for shoulder taper formulas.
- 7. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 8. This TCZ plan does not apply when work is being performed in the middle lane(s) of a six or more lane highway. See Index 102-614.
- 9. For general TCZ requirements and additional information, refer to Index 102-600.

Table I				
Device Spacing				
Max. Distance Between Devices (ft.)				
Speed (mph)	Cones or Tubular Markers		Type I or Type II  Barricades or Vertic  Panels or Drums	
	Taper	Tangent	Taper	Tangent
25	25	50	25	50
30 to 45	25	50	30	50
50 to 70	25	50	50	100

	Τá	able II	
Buffer	Space	and Ta	per Lengtl
Speed	Buffer Space	(12	er Length ' Lateral ansition)
(mph)	Dist. (ft.)	L (ft.)	Notes (Merge)
25	155	125	
30	200	180	$L = \frac{WS^2}{2}$
35	250	245	L = <u>60</u>
40	305	320	
45	360	540	
50	425	600	
55	495	660	
60	570	720	L = WS
65	645	780	
70	730	840	

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

For lateral transitions other than 12', use formula for L shown in notes column.
Where:

L = Length of taper in feet

 $W = Width \ of \ lateral \ transition \ in \ feet$ 

S = Posted speed limit (mph)

#### **DURATION NOTES**

- 1. Temporary white edgeline may be omitted for work operations less than 3 consecutive calendar days.
- 2. For work operations up to approximately 15 minutes, signs, channelizing devices, arrow board, and buffer space may be omitted if all of the following conditions are met:
- a. Speed limit is 45 mph or less.
- b. No sight obstructions to vehicles approaching the work area for a distance equal to the buffer space and the taper length combined.
- c. Volume and complexity of the roadway has been considered.
- d. The closed lane is occupied by a class 5 or larger, medium duty truck(s) with a minimum gross weight vehicle rating (GWVR) of 16,001 lb with high-intensity, rotating, flashing, oscillating, or strobe lights mounted above the cab height and operating.
- 3. For work operations up to 60 minutes, arrow board and buffer space may be omitted if conditions a, b, and c in DURATION NOTE 2 are met, and vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE LANE ADJACENT TO EITHER SHOULDER AND THE AREA 2' OUTSIDE THE EDGE OF TRAVEL WAY.

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FDOT

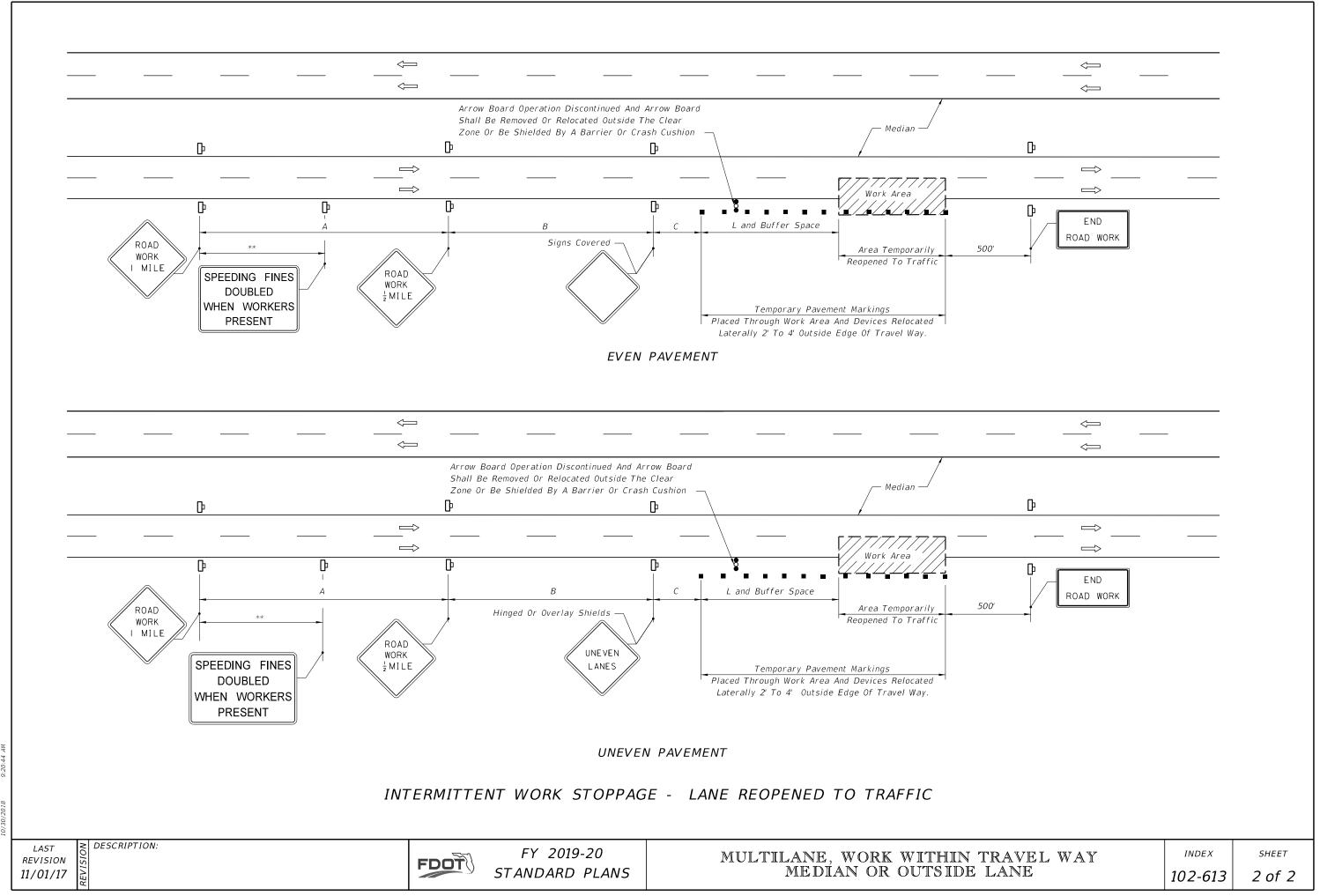
FY 2019-20 STANDARD PLANS

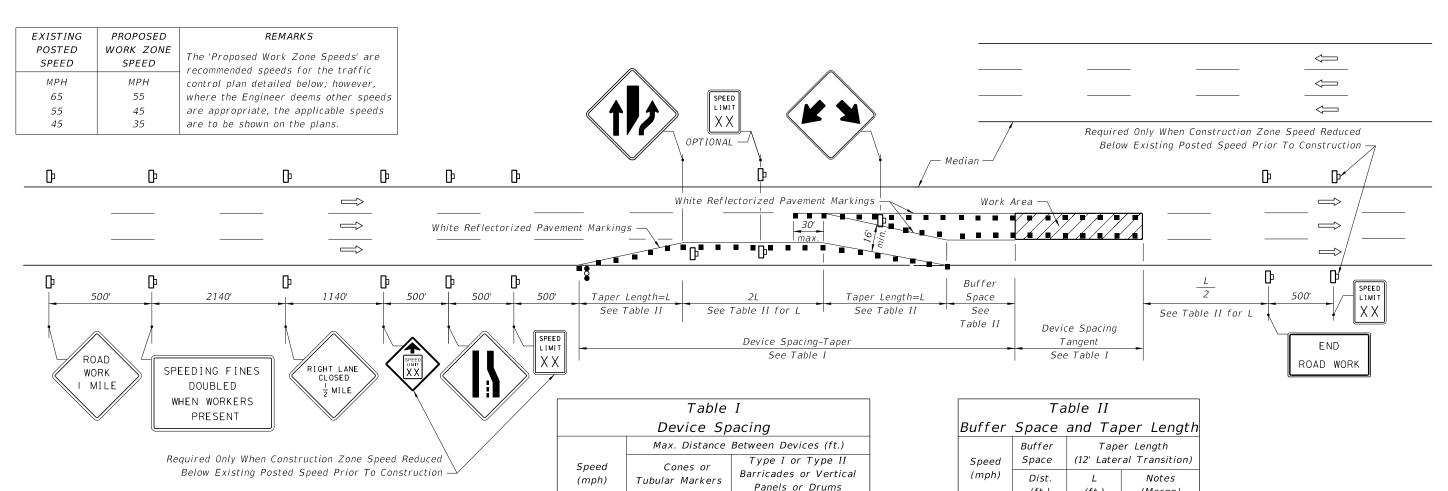
MULTILANE, WORK WITHIN TRAVEL WAY MEDIAN OR OUTSIDE LANE

INDEX

SHEET

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Tangent

50

50

100

## CONDITION NOTES

Taper

25

25

25

Tangent

50

50

50

Taper 25

30

50

- 1. The RIGHT LANE CLOSED and lane reduction signs are to be removed or fully covered when no work is being performed and the center lane is opened to traffic.
- 2. For work performed in the median or outside lane, refer to Index 102-613.

25

30 to 45

50 to 70

3. When the lane closure exceeds a continuous 24 hour period, all existing pavement markings within the realignment which conflict with the revised traffic pattern are to be removed and removable pavement marking used for marking new edge lines and centerline.

#### GENERAL NOTES

- 1. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 2. For general TCZ requirements and additional information, refer to Index 102-600.

#### **DURATION NOTES**

1. Temporary pavement markings may be omitted for work operations less than 3 days.

Table II				
Buffer	Space	and Ta _l	per Length	
Speed	Buffer Space		er Length al Transition)	
(mph)	Dist. (ft.)	L (ft.)	Notes (Merge)	
25	155	125		
30	200	180	, _ WS ²	
35	250	245	$L = \frac{110}{60}$	
40	305	320		
45	360	540		
50	425	600		
55	495	660	L = WS	
60	570	720		
65	645	780		
70	730	840		

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

For lateral transitions other than 12', use formula for L shown in the notes column. Where:

- L = Length of taper in feet
- W = Width of lateral transition in feet
- S = Posted speed limit (mph)

#### **CONDITIONS**

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON ANY PORTION OF A CENTER LANE OF A MULTILANE HIGHWAY, AND TWO DRIVING LANES ARE MAINTAINED ON THE TRAVEL

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FDOT

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MULTILANE, WORK WITHIN TRAVEL WAY, CENTER LANE INDEX

102-614 1 of 2

SHEET

DESCRIPTION:

SYMBOLS

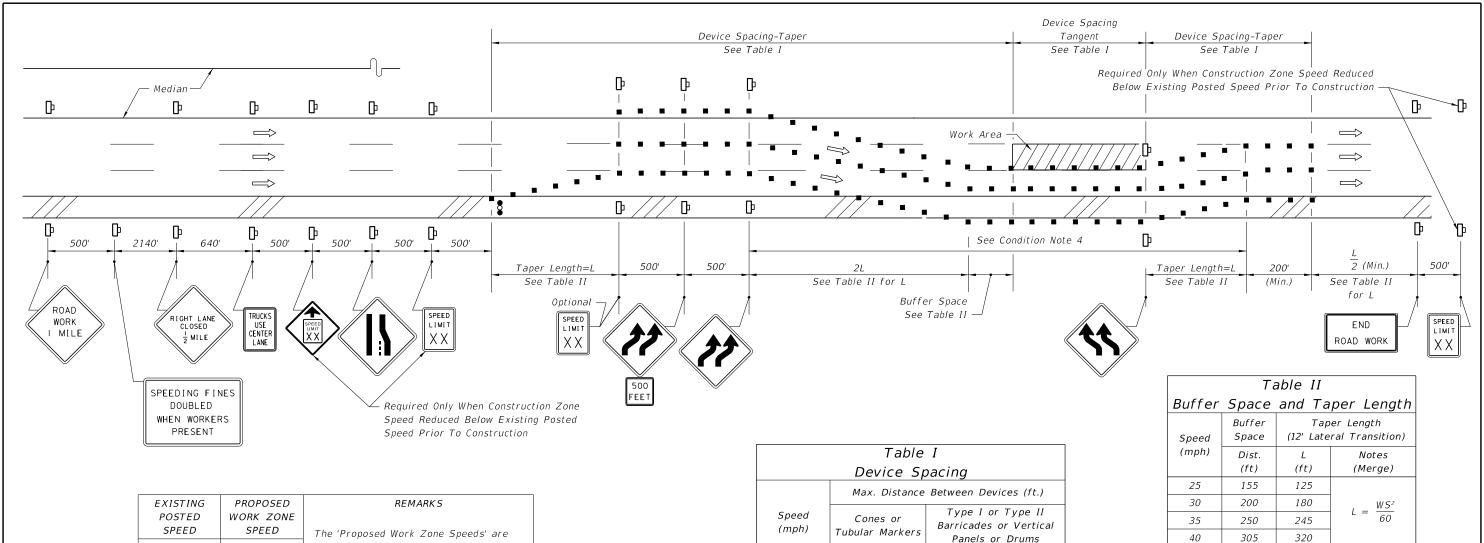
Work Zone Sign

Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic

Advance Warning Arrow Board

Work Area



Taper

25

25

25

25

30 to 45

50 to 70

Tangent

50

50

50

Taper

25

30

50

Tangent

50

50

100

#### CONDITION NOTES

1. See General Notes, Sheet 1.

recommended speeds for the traffic

control plan detailed below; however,

where the Engineer deems other speeds

are appropriate, the applicable speeds.

- 2. Length of time that traffic is using shoulder should be minimized. For example, remove lane closure and lane shift at night (unless performing night work) if practical.
- 3. The RIGHT LANE CLOSED, lane reduction and reverse curve signs are to be removed or fully covered when no work is being performed and the travel way is open to traffic.
- 4. When the lane closure exceeds a continuous 24 hour period, all existing pavement markings within the realignment which conflict with the revised traffic pattern are to be removed and removable pavement markings used for marking new edge lines and centerlines.
- 5. For general TCZ requirements and additional information, refer to Index 102-600.

#### 40 305 320 45 360 540 50 425 600 55 495 660 L = WS60 570 720

780

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

645 730

For lateral transitions other than 12', use formula for L shown in the notes column. Where:

L = Length of taper in feet

W = Width of lateral transition in feet

S = Posted speed limit (mph)

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON ANY PORTION OF A CENTER LANE OF A MULTILANE HIGHWAY, AND TWO DRIVING LANES ARE MAINTAINED, AND, THE OUTSIDE SHOULDER PAVEMENT IS TEMPORARILY USED AS A TRAVEL LANE.

65

70

## SYMBOLS

Work Area

Channelizing Device (See Index 102-600)

MPH

65

55

45

MPH

55

45

35

Work Zone Sign

Advance Warning Arrow Board

DESCRIPTION:

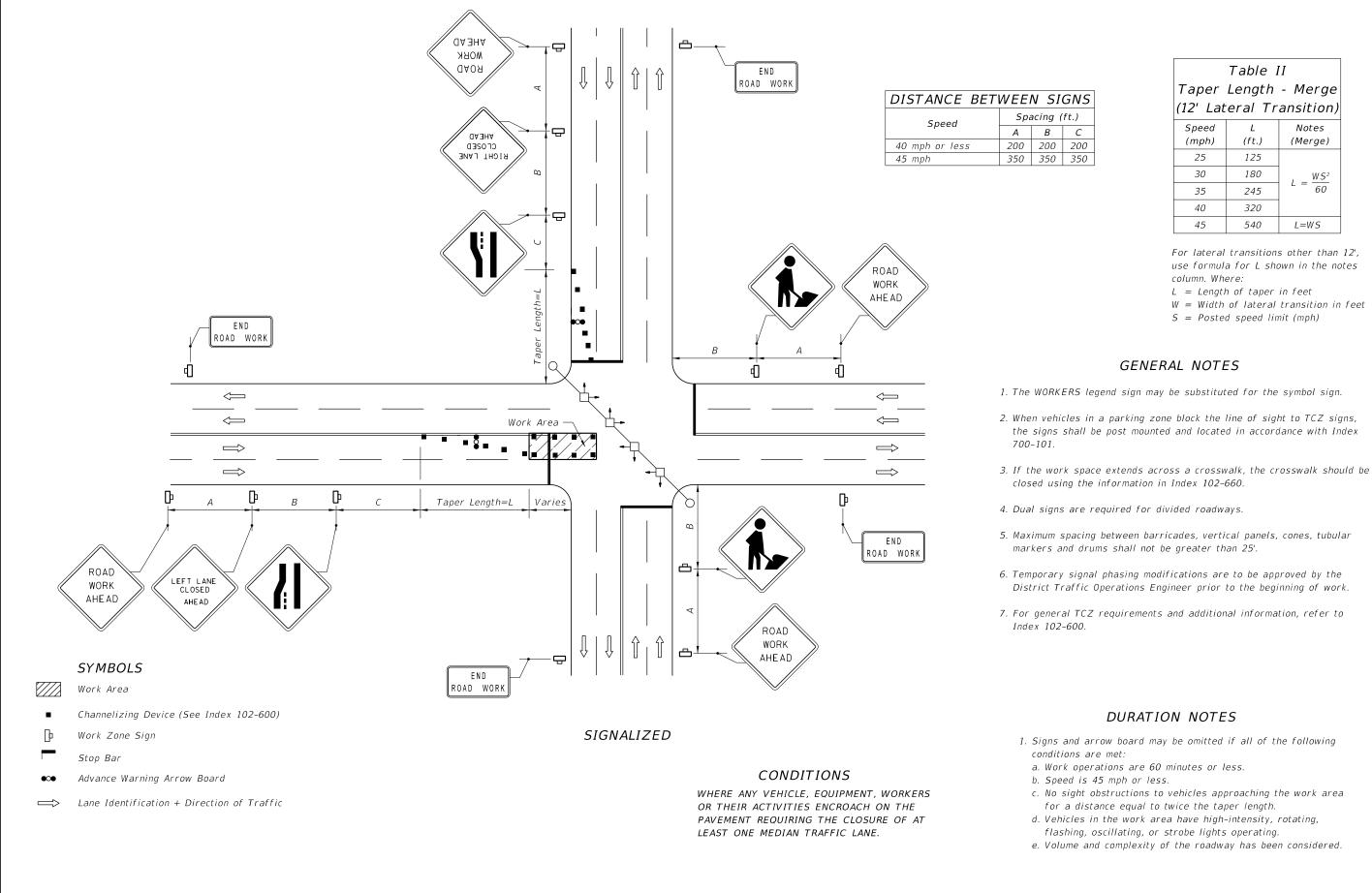
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FY 2019-20 FDOT STANDARD PLANS

MULTILANE, WORK WITHIN TRAVEL WAY, CENTER LANE INDEX

SHEET

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DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

INDEX

Table II

(ft.)

125

180

245

320

540

25

30

35

40

45

(Merge)

L=WS

SHEET

- 1. Work operations shall be confined to either one lane, or lane combinations as follows:
- a. Outside travel lane;
- b. Outside auxiliary lane;
- c. Outside travel lane and adjoining auxiliary lane;
- d. Inside travel lane  $\triangle$ ;
- e. Inside auxiliary lane △;
- f. Inside travel lane and adjoining auxiliary lane  $\triangle$
- ∧ See Sheet 3

If the work area is confined to an auxiliary lane the work area shall be barricaded and the RIGHT (LEFT) LANE CLOSED AHEAD signs replaced by ROAD WORK AHEAD signs, and the merge symbol signs eliminated.

- 2. When vehicles in a parking zone block the line of sight to TCZ signs, the signs shall be post mounted and located in accordance with Index 700-101
- 3. If the work space extends across a crosswalk, the crosswalk should be closed using the information in Index 102-660.
- 4. Signs are required on the median side for divided highways.
- 5. The two channelizing devices directly in front and directly at the end of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 6. For general TCZ requirements and additional information, refer to Index 102-600.

#### SYMBOLS



Work Area



Advance Warning Arrow Board

Type III Barricade

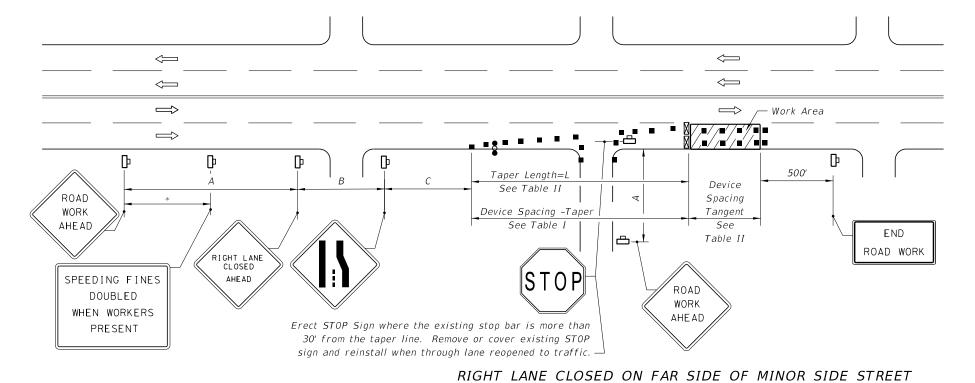
Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic

#### **DURATION NOTES**

- 1. For work operations up to approximately 15 minutes, signs, channelizing devices, and arrow board may be omitted if all of the following conditions are met:
- a. Speed limit is 45 mph or less.
- b. No sight obstructions to vehicles approaching the work area for a distance equal to twice the taper length.
- c. Volume and complexity of the roadway has been considered.
- d. The closed lane is occupied by a class 5 or larger, medium duty truck(s) with a minimum gross weight vehicle rating (GWVR) of 16,001 lb with high-intensity, rotating, flashing, oscillating, or strobe lights mounted above the cab height and operating.
- 2. For work operations up to 60 minutes, the arrow board may be omitted if conditions a, b, and c in DURATION NOTE 1 are met, and vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.

1 of 3



DISTANCE BET	WEE	N SI	GNS
Sneed	Spa	acing (	ft.)
Speed	Α	В	С
40 mph or less	200	200	200
45 mph	350	350	350
	Speed 40 mph or less	Speed	A B 40 mph or less 200 200

* 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

	Table I			
	Dev	ice Sp	pacing	
	Max.	Distance	Between Dev	rices (ft.)
Speed (mph)	Cones or Tubular Markers		Type I or Type II  Barricades or Vertical  Panels or Drums	
	Taper	Tangent	Taper	Tangent
25	25	50	25	50
30 to 45	25	50	30	50

	ROAD WORK AHE AD
$\leftarrow$	<b>←</b>
—— — — — — — — — — — — — — — — — — — —	
$\Rightarrow$	
ROAD WORK AHEAD  SPEEDING FINES DOUBLED WHEN WORKERS PRESENT  PIO	Device Spacing Tangent 500' See Table I  RIGHT LANE MUST TURN RIGHT  TURN RIGHT  TURN RIGHT  TORN RIGHT  ROAD WORK  AHE AD
	GHT LANE CLOSED ON FAR SIDE OF INTERSECTION WITH SIGNIFICANT RIGHT TURNING MOVEMENTS

Table II Taper Length - Merge (12' Lateral Transition)				
Speed	L	Notes		
(mph)	(ft)	(Merge)		
25	125			
30	180	$I = \frac{WS^2}{}$		
35	245	60		
40	320			
45	540	L=WS		

For lateral transitions other than 12', use formula for L shown in the notes column. Where:

L = Length of taper in feet

W = Width of lateral transition in feet

S = Posted speed limit (mph)

1. The normal procedure is to close on the near side of the intersection any lane that is not carried through the intersection. However, when this results in the closure of a right lane having significant right turning movements, then the right lane may be restricted to right turns only as shown in this detail.

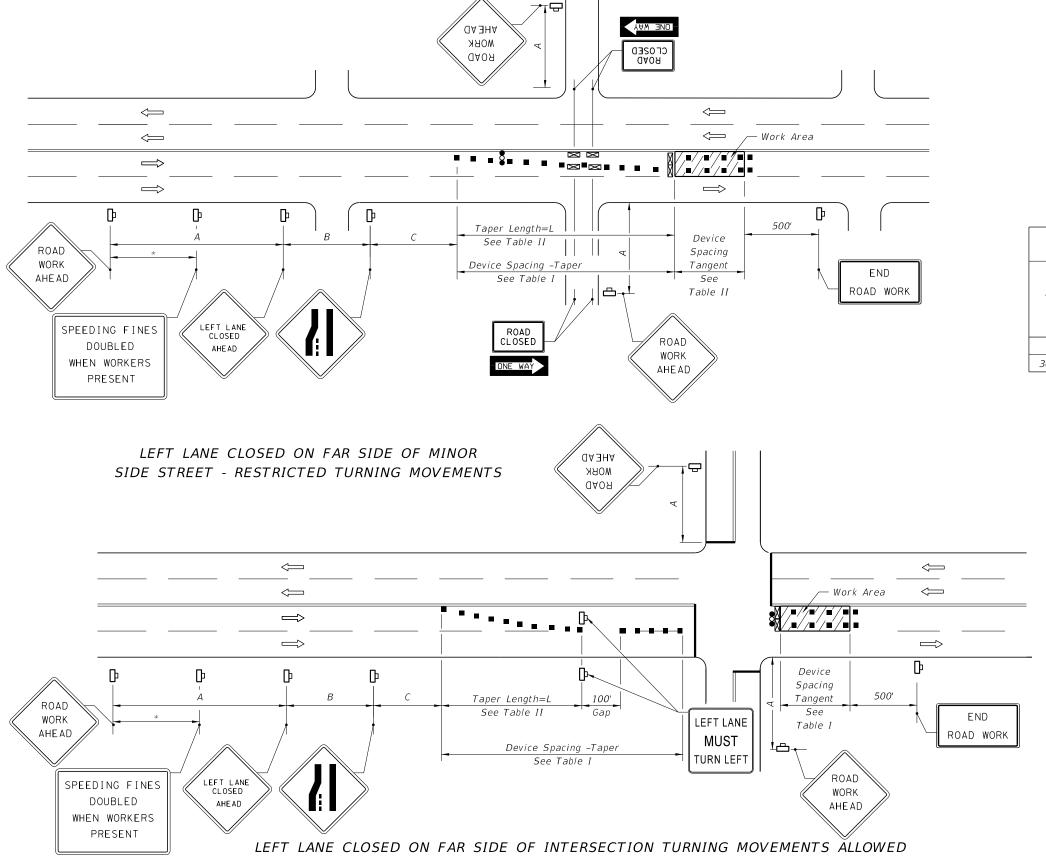
2. For intersection approaches reduced to a single lane, left turning movements may be prohibited to maintain capacity for through vehicular traffic.

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS be prohibited to maintain capacity for through vehicular traffic.



DISTANCE BET	WEE	N SI	GNS
Speed	Spacing (ft.)		
Speed	Α	В	С
40 mph or less	200	200	200
45 mph	350	350	350

* 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

Table I					
Device Spacing					
	Max.	Distance	Between Dev	vices (ft.)	
Speed (mph)	Cones or Tubular Markers Taper Tangent		Type I o Barricades Panels o	or Vertical	
			Taper	Tangent	
25	25	50	25	50	
30 to 45	25	50	30	50	

Table II						
Taper Length - Merge						
(12' La	teral Ti	ransition)				
Speed	L	Notes				
(mph)	(ft.)	(Merge)				
25	125					
30	180	$L = \frac{WS^2}{60}$				
35	245	60				
40	320					
45	540	L = WS				

For lateral transitions other than 12', use formula for L shown in the notes column. Where:

L = Length of taper in feet

W = Width of lateral transition in feet

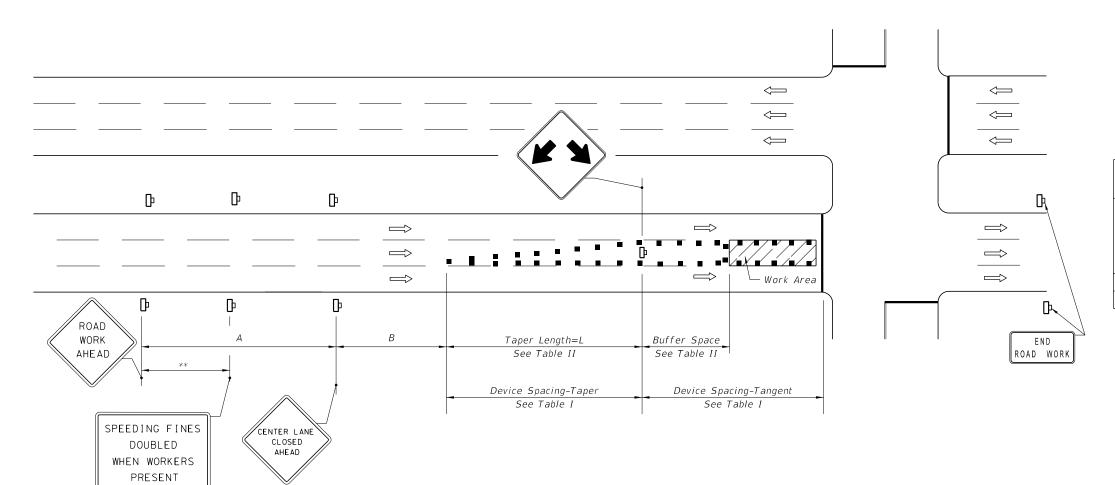
S = Posted speed limit (mph)

1. The normal procedure is to close on the near side of the intersection any lane that is not carried through the intersection. However, when this results in the closure of a left lane having significant left turning movements, then the left lane may be reopened as a turn bay for left turns only as show in this detail.

LAST REVISION 11/01/17

DESCRIPTION:

FDOT



DISTANCE BETWE	EN S	IGNS
Sneed	Spacing (ft.)	
Specu	Α	В
40 mph or less	200	200
45 mph	350	350
	Speed 40 mph or less	Speed A 40 mph or less 200

Table I					
	De	vice S	pacing		
	Max.	Distance	Between Dev	vices (ft.)	
Speed (mph)	Cones or Tubular Markers		Barricades or Vertica Type I or Type II Panels or Drums		
	Taper	Tangent	Taper	Tangent	
25	25	50	25	50	
30 to 45	25	50	30	50	

Table II Buffer Space and Taper Length						
Speed	er Length ral Transition)					
(mph)	Dist. (ft.)	L (ft.)	Notes (Merge)			
25	155	125				
30	200	180	$L = \frac{WS^2}{60}$			
35	250	245	L = <u>60</u>			
40	305	320				
45	360	540	L = WS			

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

For lateral transitions other than 12', use formula for L shown in the notes column.

L = Length of taper in feet

W = Width of lateral transition in feet

S = Posted speed limit (mph)

#### **DURATION NOTES**

- 1. Signs and buffer space may be omitted if all of the following conditions are met:
- a. Work operations are 60 minutes or less.
- b. Speed limit is 45 mph or less.
- c. No sight obstructions to vehicles approaching the work area for a distance equal to the buffer space and the taper length combined.
- d. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- e. Volume and complexity of the roadway has been considered.

## SYMBOLS



■ Channelizing Device (See Index 102-600)

** 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

- Work Zone Sign
- Advance Warning Arrow Board

DESCRIPTION:

#### GENERAL NOTES

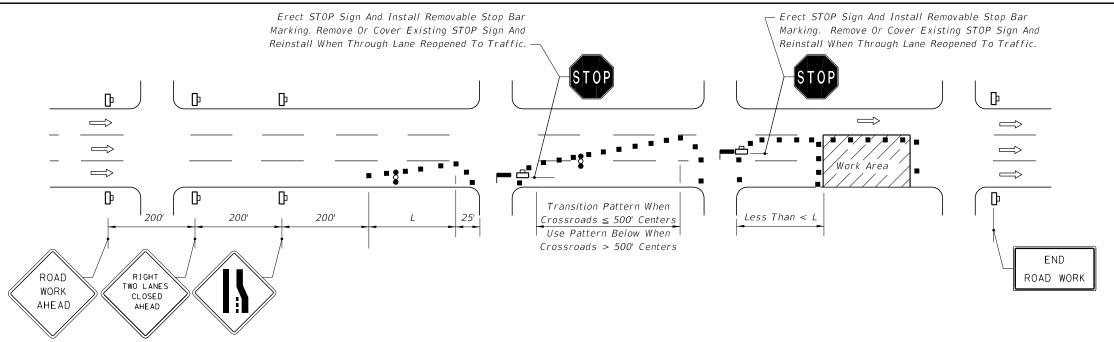
- 1. Work operations shall be confined to one center travel lane, leaving the adjacent travel lanes open to traffic.
- 2. The merging taper shall direct vehicular traffic into either the right or left lane, but not both.
- 3. When vehicles in a parking zone block the line of sight to TCZ signs, the signs shall be post mounted and located in accordance with Index 700-101.
- 4. If the work space extends across a crosswalk, the crosswalk should be closed using the information in Index 102-660.
- 5. For general TCZ requirements and additional information, refer to Index 102-600.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF THE CENTER LANE NEAR AN INTERSECTION.

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FDOT



#### **CONDITIONS**

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF EITHER THE OUTSIDE AND CENTER TRAVEL LANES OR THE MEDIAN AND CENTER TRAVEL LANES, WITH OR WITHOUT CLOSURE OF ADJOINING AUXILIARY LANES, FOR WORK AREA LESS THAN 200' FROM INTERSECTION, FOR A PERIOD OF MORE THAN 60 MINUTES.

#### **CONDITIONS**

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF EITHER THE OUTSIDE AND CENTER TRAVEL LANES OR THE MEDIAN AND CENTER TRAVEL LANES, WITH OR WITHOUT CLOSURE OF ADJOINING AUXILIARY LANES, FOR WORK AREA 200' OR MORE FROM INTERSECTION, FOR A PERIOD OF MORE THAN 60 MINUTES.

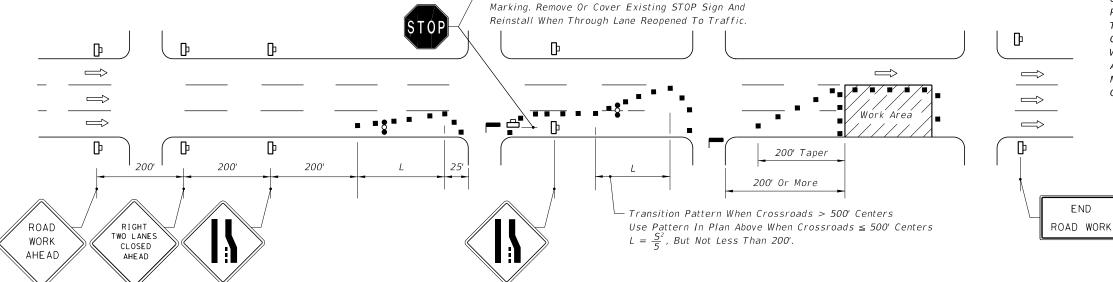


For lateral transitions other than 12', use formula for L shown in the notes

- L = Length of taper in feet
- W = Width of lateral transition in feet
- S = Posted speed limit (mph)

1 '	_	II - Merge ransition)
Speed	L	Notes
(mph)	(ft.)	(Merge)
25	125	
30	180	$L = \frac{WS^2}{60}$
35	245	60
40	320	
45	540	L = WS

column. Where:



— Erect STOP Sign And Install Removable Stop Bar

### GENERAL NOTES

- 1. If the work space extends across a crosswalk, the crosswalk should be closed using the information in
- 2. Signs are required on the median side for divided highways.
- 3. The two channelizing devices directly in front and directly at the end of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 4. Within the lateral transitions, the maximum spacing between cones and tubular markers shall be 25'. Maximum spacing between Type I or Type II barricades or vertical panels or drums shall be based on the speed limit as follows: 15' up to 25 MPH; 30' for 30-40 MPH; 50' for 45 MPH.

Spacing for devices parallel to the travel lanes shall be 25' centers for cones or tubular markers and 50' centers for Type I or Type II barricades or vertical panels or drums for 250', thereafter, cones or tubular markers at 50' centers and Type I  $\,$  or Type II  $\,$  barricades or vertical panels or drums at 100' centers.

5. For general TCZ requirements and additional information, refer to Index 102-600.

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Work Zone Sign

DESCRIPTION:

Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic

Advance Warning Arrow Board

Work Area

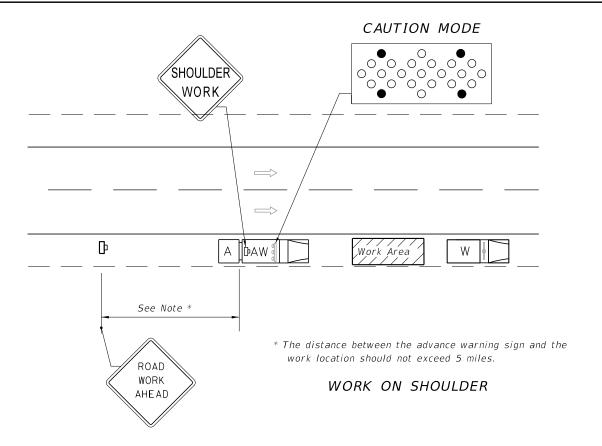
FDOT

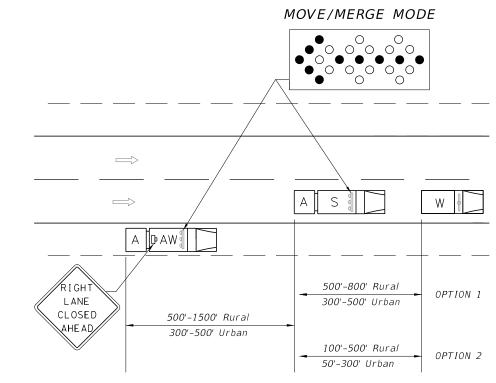
FY 2019-20 STANDARD PLANS

MULTILANE, WORK IN INTERSECTION TWO LANES CLOSED - 45 MPH OR LESS INDEX

SHEET

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OPTION 1: Advanced Warning Vehicle may be operated in the lane behind the Shadow Vehicle where adequate shoulder width is not available. Approved Truck Mounted Attenuators are required on both the Advance Warning Vehicle and the Shadow Vehicle.

OPTION 2: Advance Warning Vehicle must be operated in the lane behind the Shadow Vehicle.

Approved Truck Mounted Attenuators are required on both the Advance Warning Vehicle and the Shadow Vehicle.

WORK WITHIN TRAVEL LANE
(Option 1 Shown, Option 2 Similar)

#### GENERAL NOTES

- 1. These illustrations are representative of general conditions.
- 2. The figures illustrate closing the right shoulder or right lanes for various lane configurations. When work is required on left side of roadways, the inverted plan is to be applied. The intent of this index is to allow passing on only one side of the work convoy.
- 3. Arrow boards shall not be obscured by equipment, supplies, signs, or the enclosure.
- 4. Vehicle-mounted signs shall be mounted with the bottom of the sign at a minimum height of 48 inches above the pavement. Vehicle mounted changeable message signs may be used in lieu of truck mounted static signs. Changeable message signs shall flash alternately to read "Left or Right Lane" or "Two Left or Two Right Lanes", "Closed Ahead", and the arrow symbol. Arrow boards shall not be used with truck mounted changeable message signs. Sign legends shall be covered or turned from view when work is not in progress.
- 5. On freeway facilities (interstates, toll roads, and expressways), a traffic control officer is required for all nighttime non-emergency operations for work within the travel lane.

- 6. If the work vehicle speed exceeds the minimum legal speed limit on limited access facilities and one half the posted speed limit on other facilities, the Engineer may delete requirements for shadow vehicle and attenuator. The work vehicle will be required to have an arrow board and sign message.
- 7. Where work activities within 2' of the edge of travel way are Incidental (i.e. Mowing, Litter Removal), the Engineer may delete requirements for signs and the advance warning vehicle provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
- 8. Work, Shadow, and Advance Warning Vehicles shall have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- 9. Functional two-way communication is required between all vehicles in the mobile operation convoy.
- 10. For general TCZ requirements and additional information, refer to Index 102-600.

SYMBOLS

₩ ₩ Work Vehicle

S

Shadow (S) Vehicle with Arrow Board

PAW

Advance Warning (AW) Vehicle with Arrow Board and Sign Message or Changeable Message Sign

A

Truck/Trailer Mounted Attenuator (TMA)

 $\Longrightarrow$ 

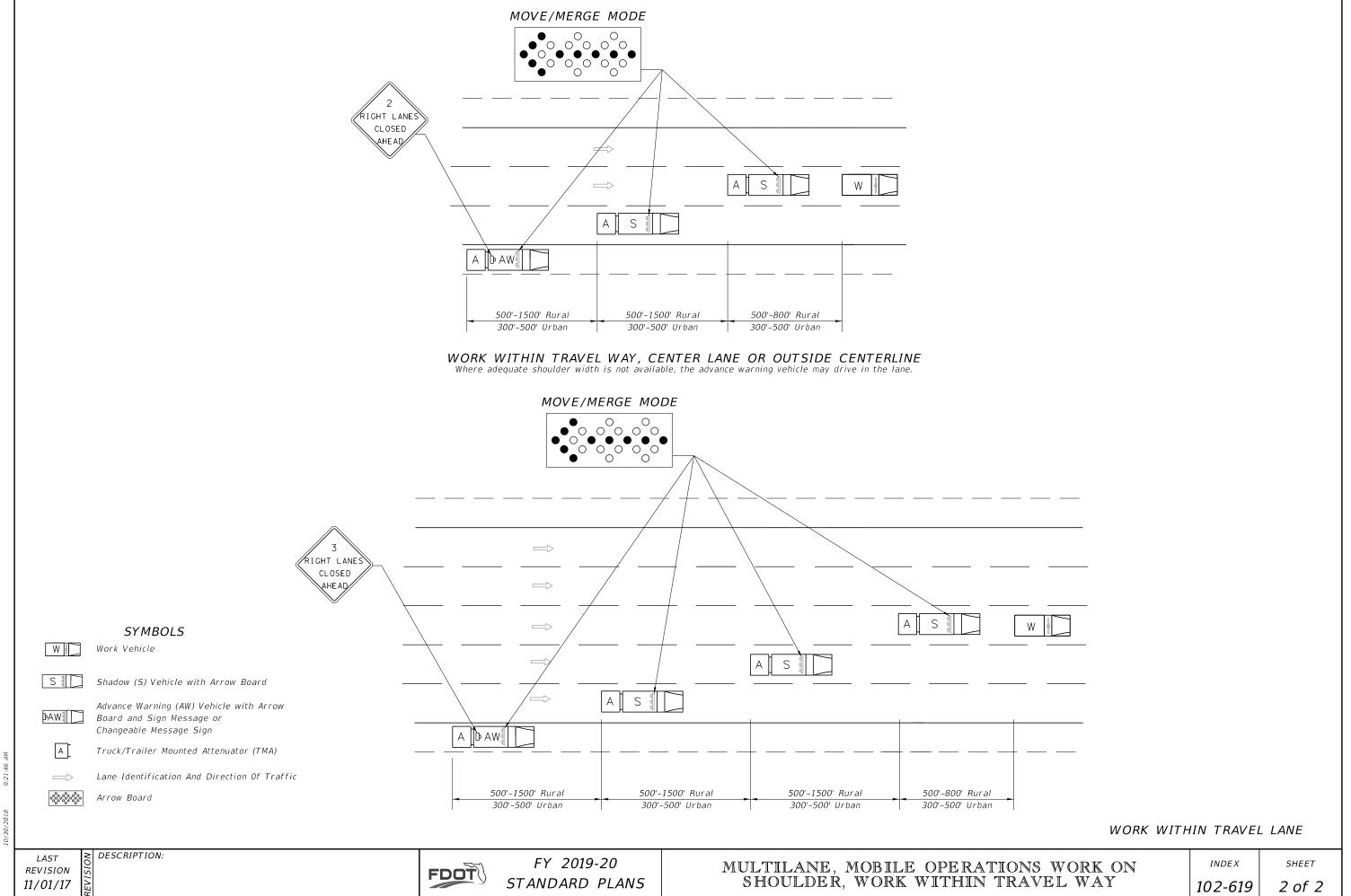
Lane Identification And Direction Of Traffic

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Arrow Board

DESCRIPTION:

LAST REVISION 11/01/17



- 1. TWO-WAY TRAFFIC sign(s) shall be repeated every  $\frac{1}{4}$  mile in each direction, throughout the tangent distance (T).
- 2. L (min.) = WS for speeds  $\geq$  45 mph  $WS^2$ 60 = --- for speeds ≤ 40 mph

Where:

W= Width of lateral transition in feet.

S= Posted speed limit (mph).

- 3. Where the tangent distance (T) exceeds 250', spacing between Type I or II barricades or vertical panels or drums may be increased to 100' within the limits of the tangent, or post mounted delineators at 50' centers may be substituted for barricades, vertical panels or drums.
- 4. All existing pavement markings within the realignment which conflict with the revised traffic pattern are to be removed and removable pavement markings used for making new edge lines.
- 5. When side roads, cross roads or interchanges intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 6. For general TCZ requirements and additional information, refer to Index 102-600.

#### SCHEME APPLICATIONS

- Scheme 1: Restricted Construction Limits.
- Scheme 2: Unrestricted Construction Limits And Light To Moderate Traffic.
- Scheme 3: Unrestricted Construction Limits And Moderate To Heavy Traffic.
  - Where: Construction Limits Are The Outward Beginning Or Ending Of Lane Reductions.
  - Where: Unless A Specific Scheme Is Called For In The Plans, Scheme Selection Shall Be At The Contractor's Option And As Approved By The Engineer.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES REQUIRE THE CLOSURE OF ONE ROADWAY AND THE OPPOSING ROADWAY IS CONVERTED TO TEMPORARY TWO-WAY TRAVEL BY WAY OF CROSSOVERS.

Work Area

Channelizing Device (See Index 102-600)

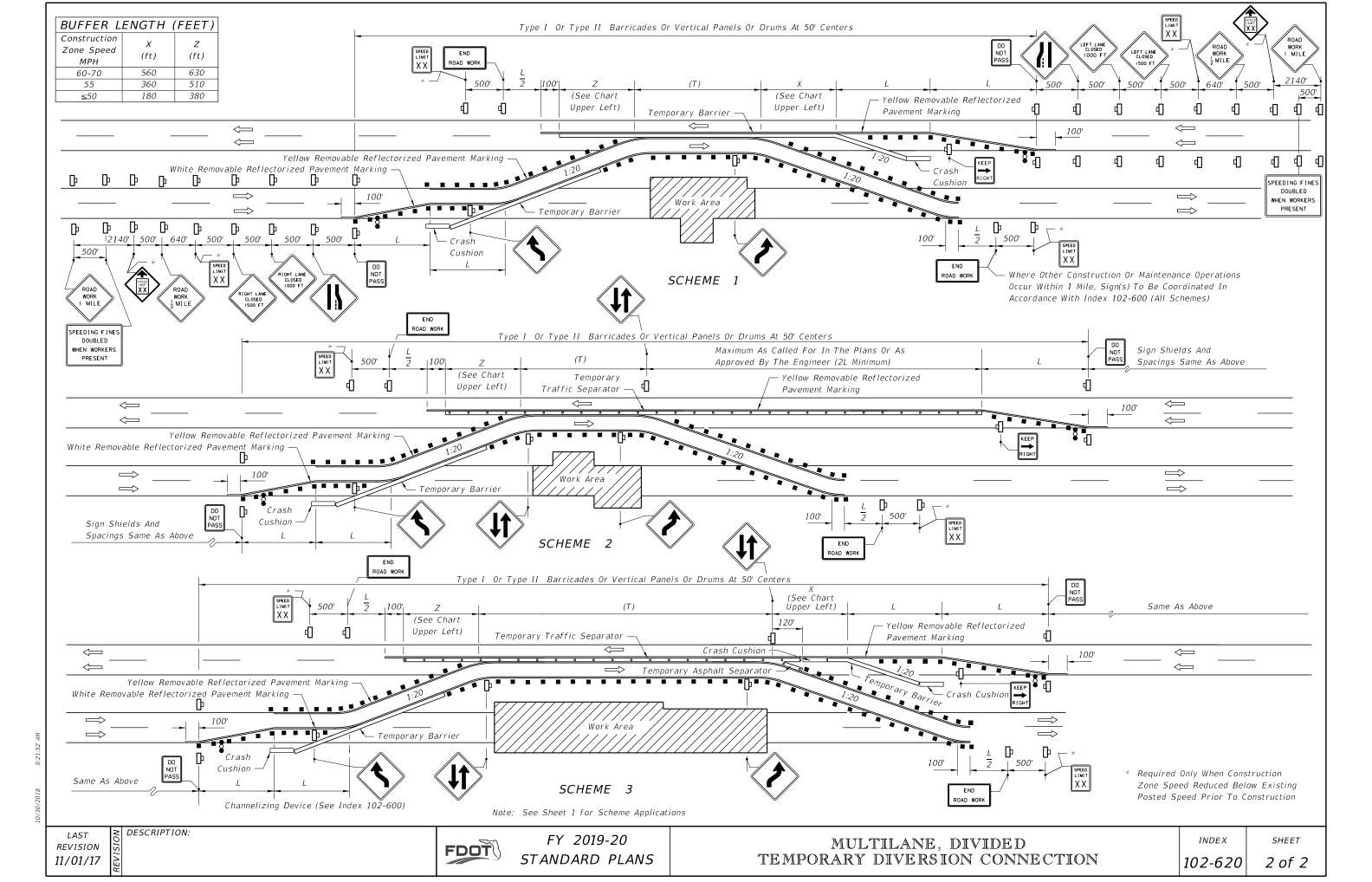
Work Zone Sign

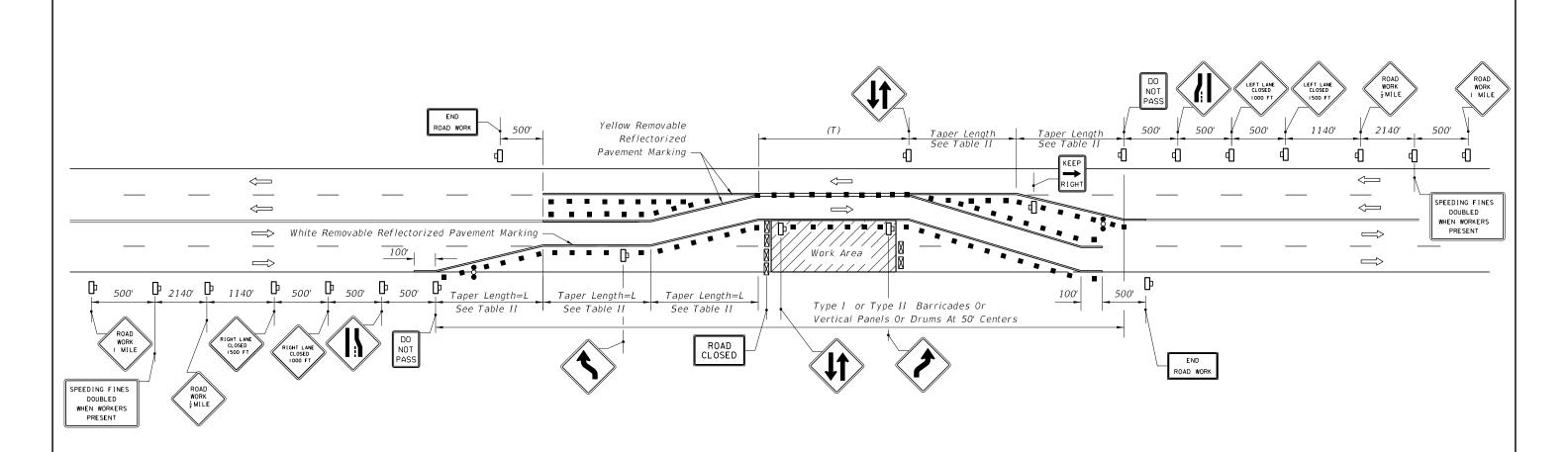
DESCRIPTION:

Advance Warning Arrow Board

Lane Identification + Direction of Traffic

**REVISION** 11/01/17





- 1. TWO-WAY TRAFFIC signs shall be repeated every  $\frac{1}{4}$  mile in each direction, through the tangent distance (T).
- 2. When paved shoulders having a width of 8 ft. or more are closed, channelizing devices shall be used to close the shoulder in advance of the merging taper to direct vehicular traffic to remain within the travel way. See Index 102-612 for shoulder taper formulas.
- 3. Where the tangent distance (T) exceeds 250', spacing between cones or tubular markers may be increased to 50' or spacing between Type I or Type II barricades or vertical panels or drums may be increased to 100' within the limits of the tangent.
- 4. This index does not apply when work is being performed in the middle lane(s) of a six or more lane highway. Special maintenance of traffic details will be required.
- 5. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 6. For general TCZ requirements and additional information, refer to Index 102-600.

Table II Taper Length - Merge (12' Lateral Transition)					
Speed (mph)	L (ft.)	Notes (Merge)			
25	125	w.c.2			
30	180	$L = \frac{WS^2}{60}$			
35	245				
40	320				
45	540				
50	600				
55	660	L=WS			
60	720	L-W3			
65	780				
70	840				

For lateral transitions other than 12' use formula for L shown in the notes column. Where:

L = Length of taper in feet

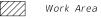
W = Width of lateral transition in feet

S = Posted speed limit (mph)

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES REQUIRE THE CLOSURE OF THE LANES IN ONE DIRECTION AND A DIVERSION IS PROVIDED BY UTILIZING ONE LANE OF THE OPPOSING TRAFFIC LANES.

## SYMBOLS



Channelizing Device (See Index 102-600)

Type III Barricade

Work Zone Sign

Advance Warning Arrow Board

Lane Identification + Direction of Traffic

**REVISION** 11/01/17

DESCRIPTION:



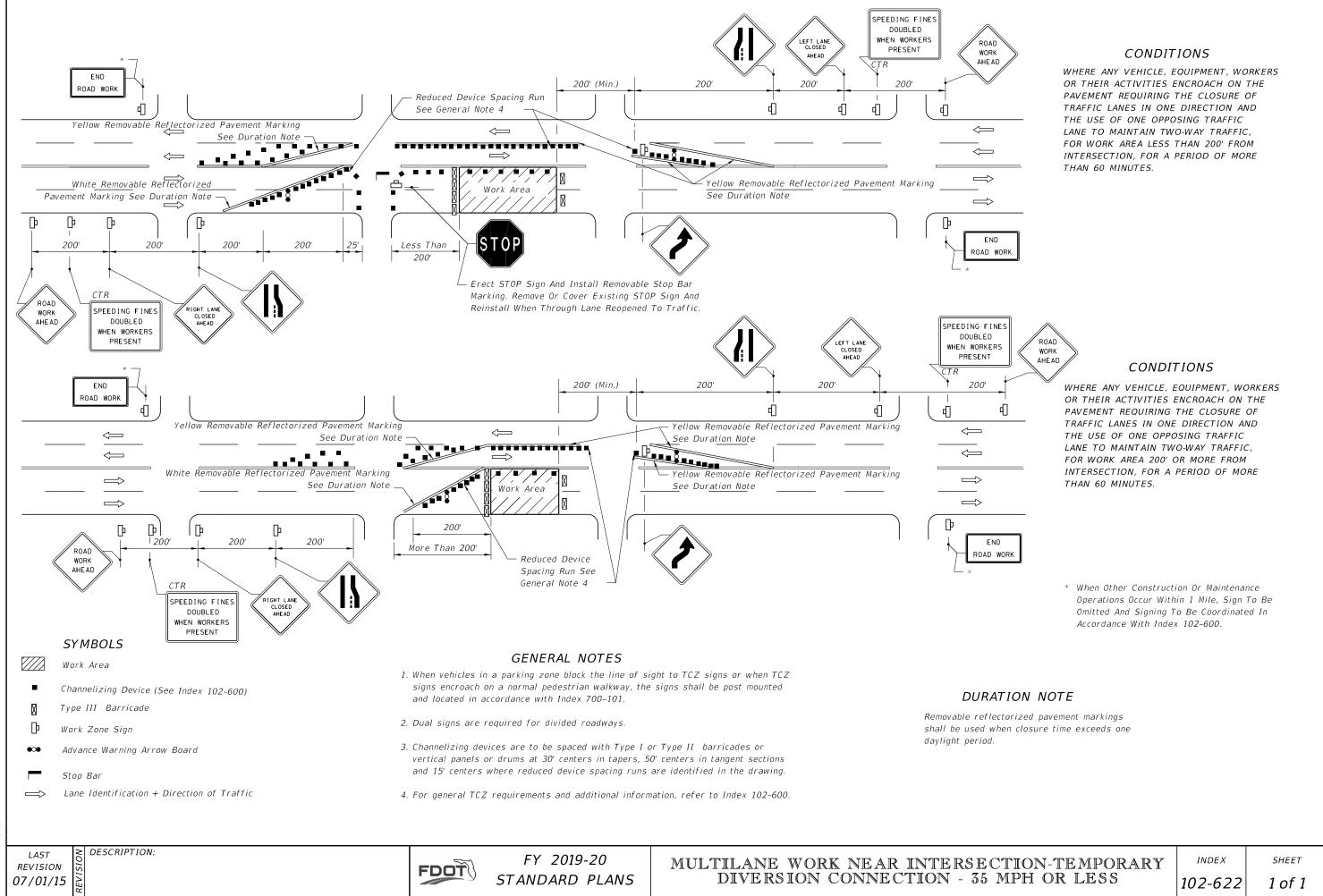
FY 2019-20

MULTILANE, UNDIVIDED TEMPORARY DIVERSION CONNECTION INDEX

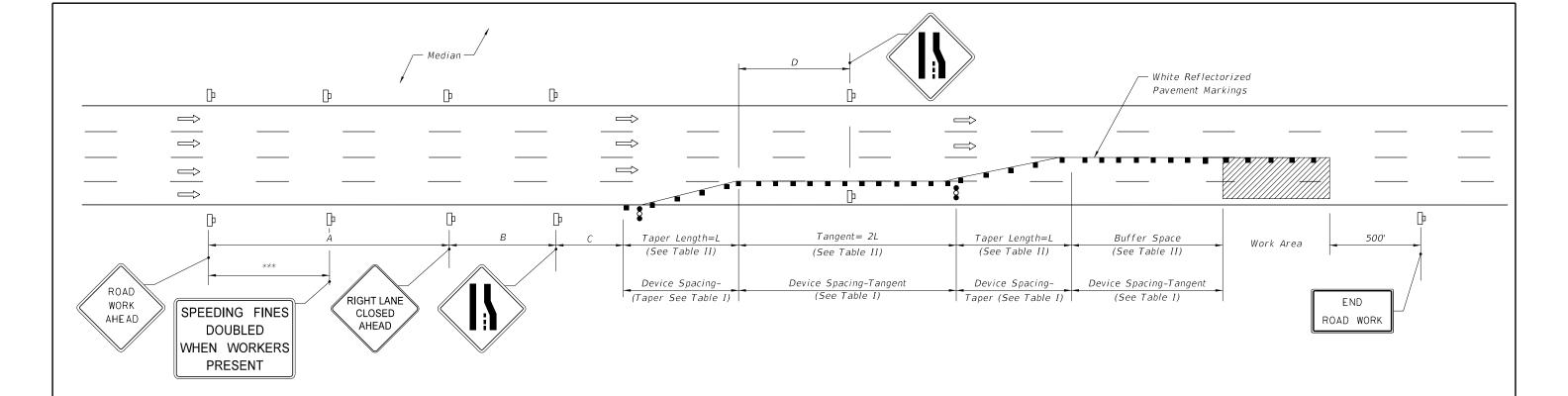
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10/30/20



DISTANCE BETWEEN SIGNS					
Enood		Spaci	ng (ft.	)	
Speed	Α	В	С	D**	
40 mph or less	200	200	200	L	
45 mph	350	350	350	L	
50 mph	500	500	500	L	
*55 mph or greater	2640	1640	1000	L	

- * The ROAD WORK 1 MILE sign may be used as an alternate to the ROAD WORK AHEAD sign MILE sign may be used as an alternate to the RIGHT LANE CLOSED AHEAD sign.
- ** See Table II for L
- *** 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

Table I						
	Device Spacing					
	Max.	Distance	Between D	evices (ft.)		
Speed (mph)	Cones or Tubular Markers		Type I or Type II Barricades or Vertical Panels or Drums			
	Taper	Tangent	Taper	Tangent		
25	25	50	25	50		
30 to 45	25	50	30	50		
50 to 70	25	50	50	100		

	Table II				
Buffe	er Spac	e and 7	aper L	ength	
Buffer Taper Length (12' Lateral Speed Space Transition)			ateral	Tangent	
(mph)	Dist. (ft.)	L (ft.)	Notes (Merge)	2L (ft.)	
25	155	125	W S ^z	250	
30	200	180		360	
35	250	245	$L = \frac{WS}{60}$	490	
40	305	320		640	
45	360	540		1080	
50	425	600		1200	
55	495	660		1320	
60	570	720	L = WS	1440	
65	645	780		1560	
70	730	840		1680	

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

For lateral transitions other than 12', use formula for L shown in the notes column. Where:

L= Length of taper in feet

W= Width of lateral transition in feet

S= Posted speed limit (mph)

#### GENERAL NOTES

- 1. Work operations shall be confined to the two outside traffic lanes, leaving the adjacent lane(s) open to traffic.
- 2. On undivided highways the median signs as shown are to be
- 3. When work is performed in the median lane on divided highways, the channelizing device plan is inverted and left lanes closed and lane ends signs substituted for the right lanes closed and lane end signs.
- 4. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 5. For general TCZ requirements and additional information, refer to Index 102-600.
- 6. When paved shoulders having a width of 8 ft. or more are closed, channelizing devices shall be used to close the shoulder in advance of the merging taper to direct vehicular traffic to remain within the travel way. See Index 102-612 for shoulder taper formulas.

#### **DURATION**

Temporary white edgeline may be omitted for work operations less than three (3) days.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE TWO LANES ADJACENT TO EITHER SHOULDER.

### SYMBOLS



11/01/17

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

Advance Warning Arrow Board

DESCRIPTION: **REVISION** 

FDOT

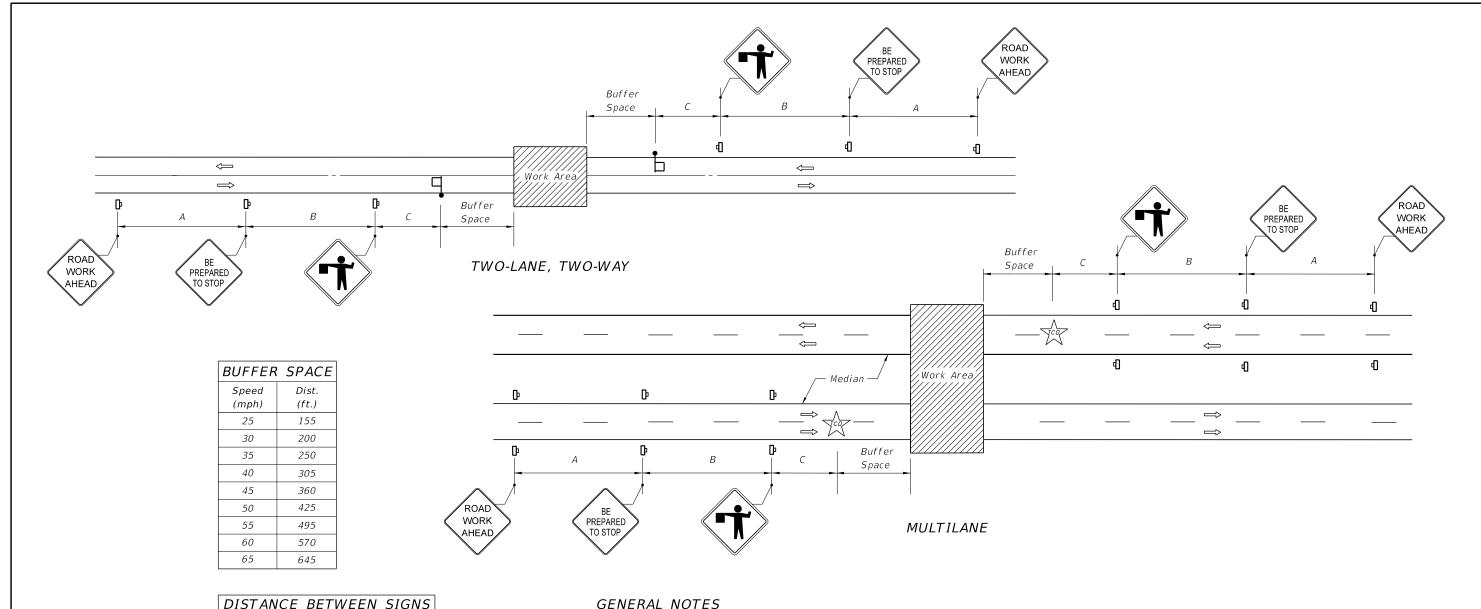
FY 2019-20 STANDARD PLANS

MULTILANE, WORK WITHIN THE TRAVEL WAY

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(mph) A B C 40 or less 200 200 200 **SYMBOLS** 45 350 350 350 50 or greater 500 500 500

Speed

Spacing (ft.)

Work Area

Work Zone Sign

Flagger

Traffic Control Officer

Lane Identification + Direction of Traffic

DESCRIPTION:

#### GENERAL NOTES

- 1. This Index does not apply to limited access facilities.
- 2. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with applicable TCZ Indexes.
- 3. Traffic volume or complexity of the roadway may dictate additional devices, signs, flagmen and/or a traffic control officer.
- 4. The buffer space may be omitted if there are no sight obstructions to vehicles approaching the Flagger/Officer for distance equal to the buffer space.
- 5. A Flagger may be substituted for a Traffic Control Officer and the BE PREPARED TO STOP sign may be omitted, when the following conditions are met:
- a. Speed limit is 45 mph or less.
- b. No sight obstructions to vehicles approaching the Flagger/Officer for a distance equal to the buffer space.
- c. Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
- 6. On undivided highways the median sign as shown are to be omitted.

7. For general TCZ requirements and additional information refer to Index 102-600.

CONDITIONS

PLANNED CLOSURE NOT EXCEEDING 5 MINUTES

**REVISION** 11/01/17

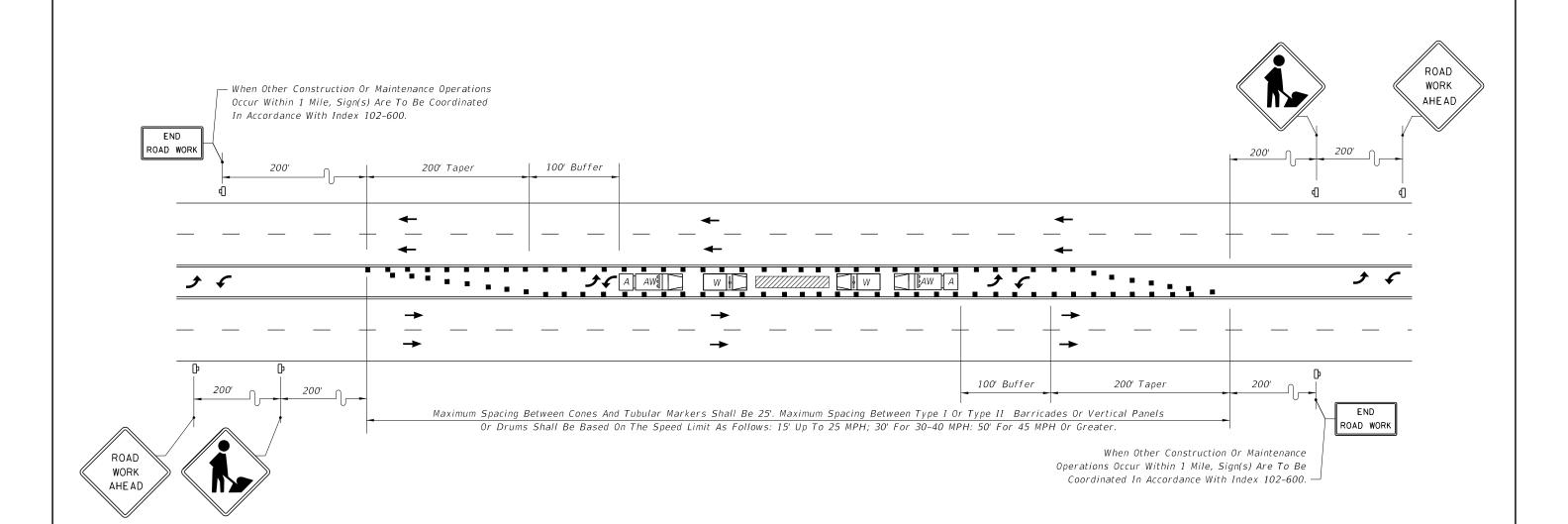
FDOT

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#### **SYMBOLS**

Work Area

Channelizing Device (See Index 102-600)

₩ork Zone Sign

DESCRIPTION:

Work Vehicle With Rotating/Strobe Lights



Shadow (S) Or Advance Warning (AW) X Vehicle with Advance Warning Arrow Board and Sign Message

Truck/Trailer Mounted Attenuator (TMA)

#### GENERAL NOTES

- 1. Work operations shall be confined to two way left turn lane, leaving the adjacent lanes open to traffic.
- 2. Advance Warning Vehicle will have an Advanced Warning Arrow Board in the Warning Mode.
- 3. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
- 4. For general TCZ requirements and additional information, refer to Index 102-600.

#### CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ARE BEING CONDUCTED IN THE TWO WAY LEFT TURN LANE.

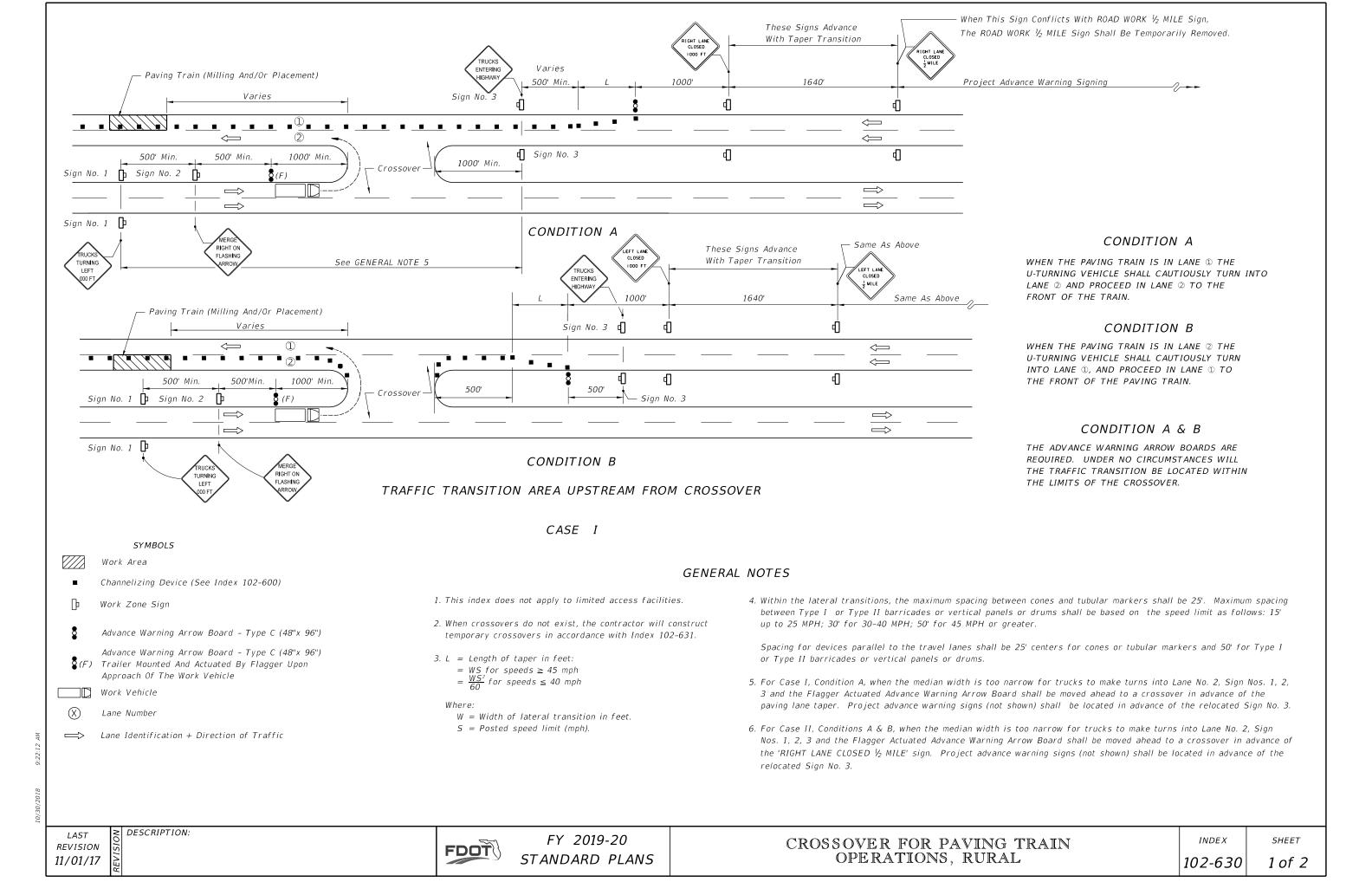
**REVISION** 11/01/17

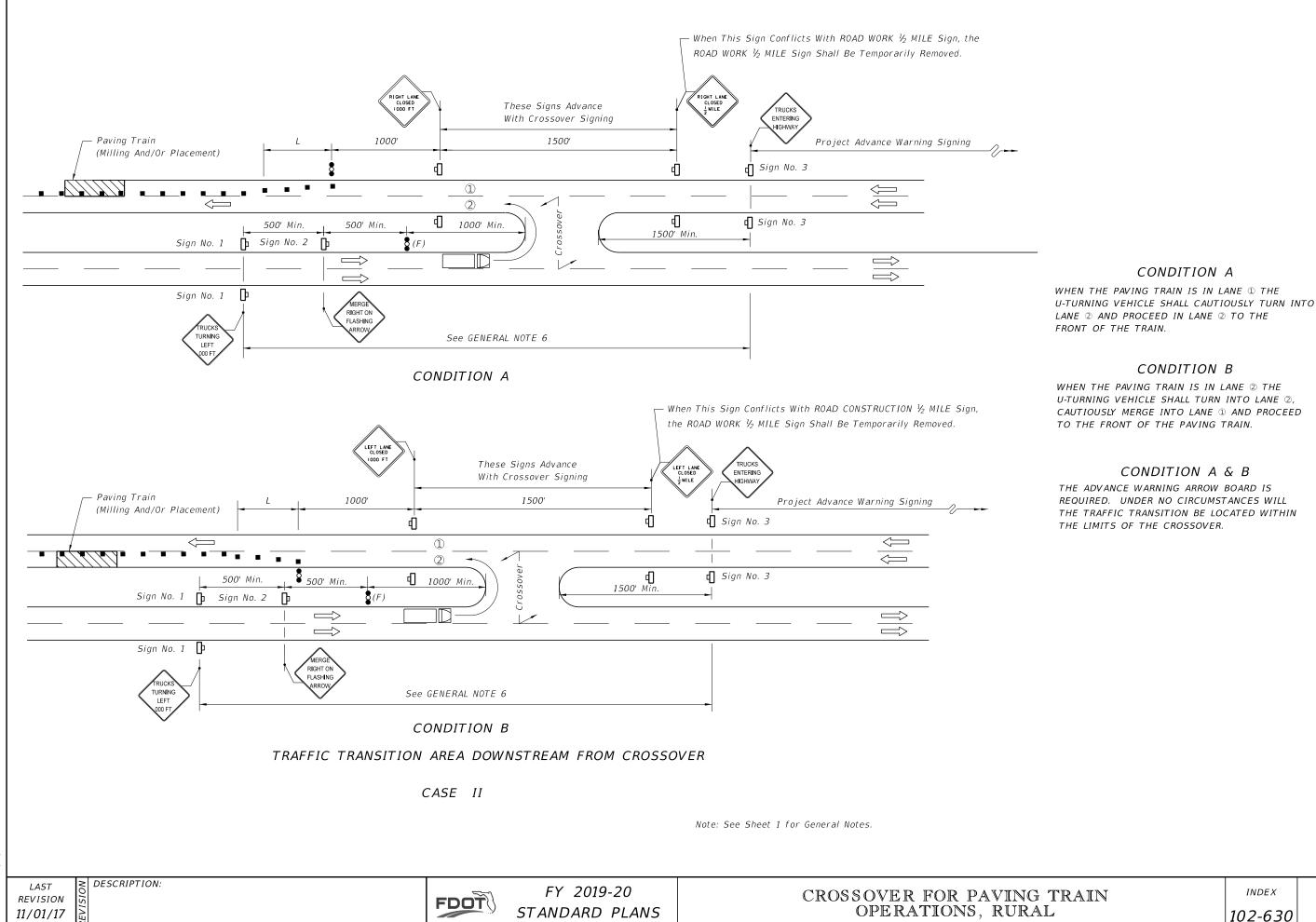
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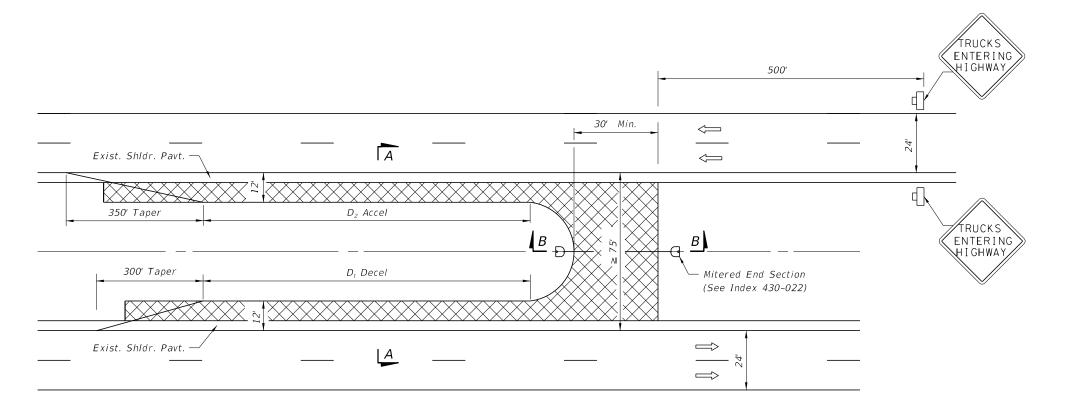
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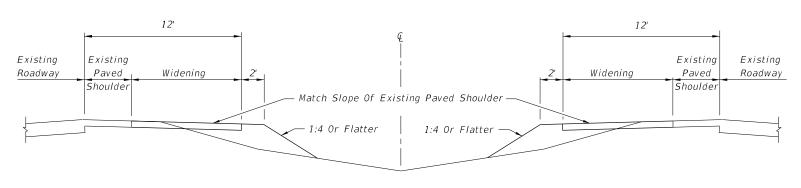




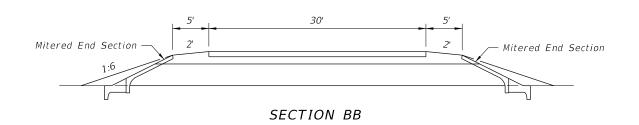


LENGTH OF ACCESS LANES (Ft.)		ES (Ft.)
Grade	D ₁	$D_2$
2% or less	590'	1540'
3 to 4% Upgrade	530'	2310'
3 to 4% Downgrade	710'	925'

#### PLAN



#### SECTION AA



#### SYMBOLS

- Work Zone Sign
- Lane Identification + Direction of Traffic
- Temporary Pavement

#### GENERAL NOTES

- 1. Temporary median crossovers shall be within the project limits and shall not be used for transporting materials to or from any other project. The acceleration-deceleration surfaces shall be paved. RAP material is acceptable for crossing surfacing.
- 2. Temporary median crossovers shall be located only in areas having adequate sight distance. On limited access facilities temporary median crossovers shall not be located within 1.5 miles of interchanges nor within 2000 ft. of acceleration-deceleration lanes at rest areas, other access openings or other highway service areas.
- 3. For paving train operations at permanent crossovers, see Index 102-630.
- 4. All traffic control devices are to be removed when crossover will not be in use for one hour or longer.
- 5. Trailer mounted advance warning panel may be used in lieu of advance warning vehicle.
- 6. When a crossover is no longer needed, all temporary construction shall be immediately removed and the area restored to its original condition.
- 7. Cost of construction, maintenance, removal and restoration work related to temporary crossovers shall be included in the contract unit price for Maintenance of Traffic, LS.
- 8. Temporary crossovers on limited access right of way and use of this Index are prohibited unless specifically permitted in the Contract Plans or Special Provisions. When permitted in the Contract Plans or Special Provisions and prior to construction of any temporary crossover, the Contractor must submit, in writing, a request identifying specific locations for approval by the Engineer.
- 9. Pipe and mitered end sections are not required when crossover is located at the high point of a crest vertical curve.

## TEMPORARY CROSSOVER FOR MEDIAN WIDTHS ≥ 75'

**REVISION** 11/01/17

DESCRIPTION:

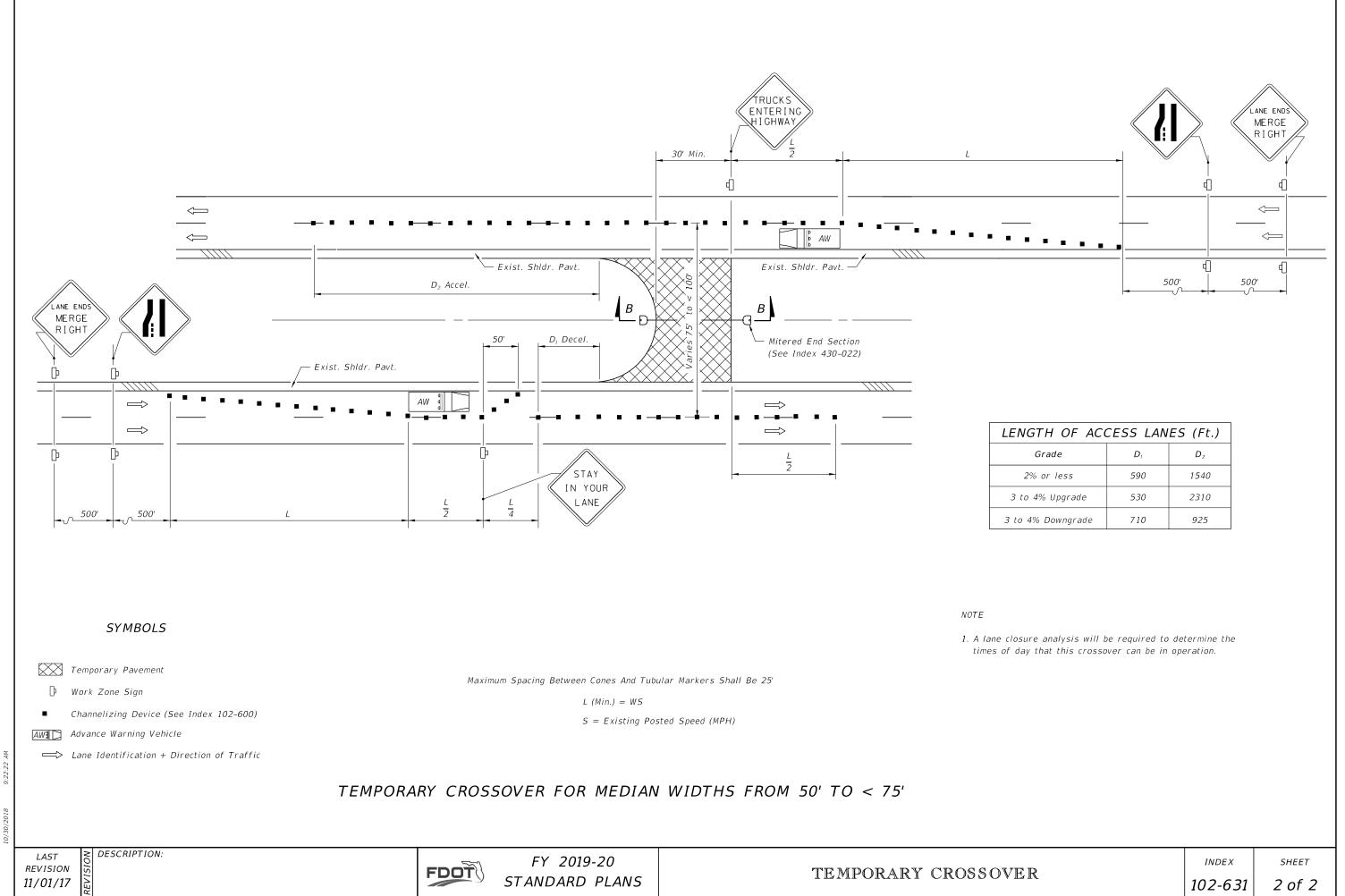


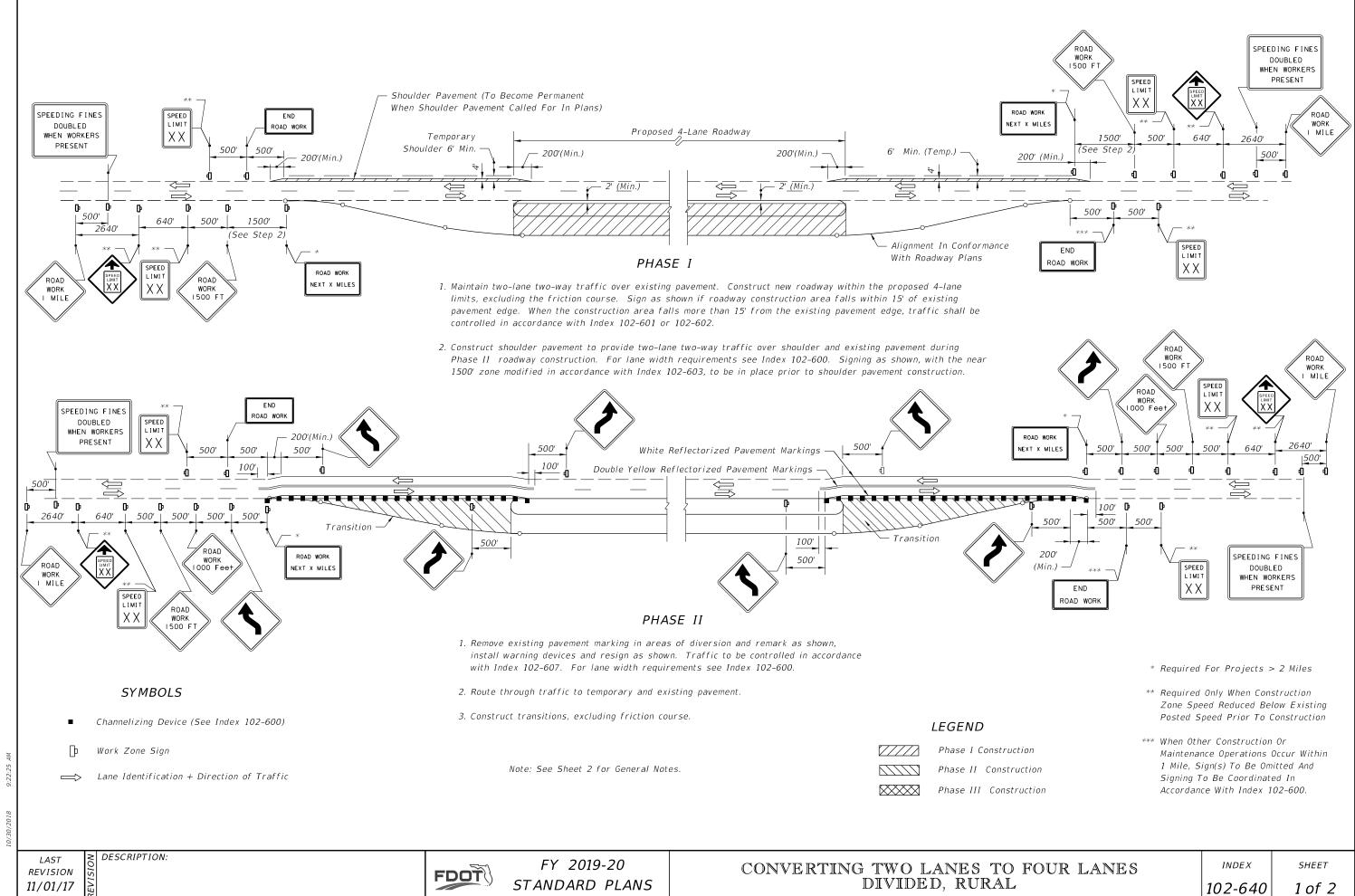
FY 2019-20 STANDARD PLANS

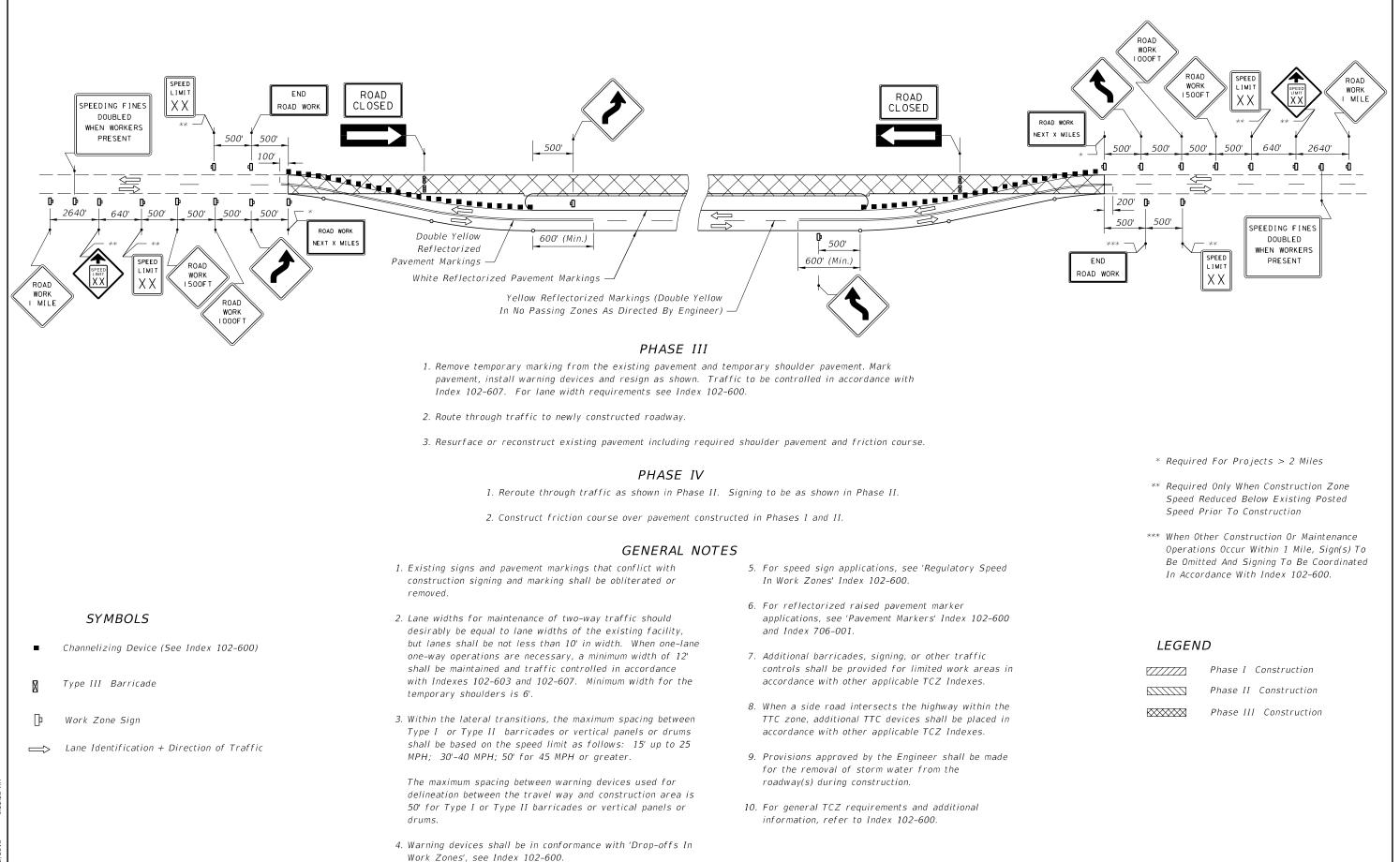
TEMPORARY CROSSOVER

INDEX 102-631

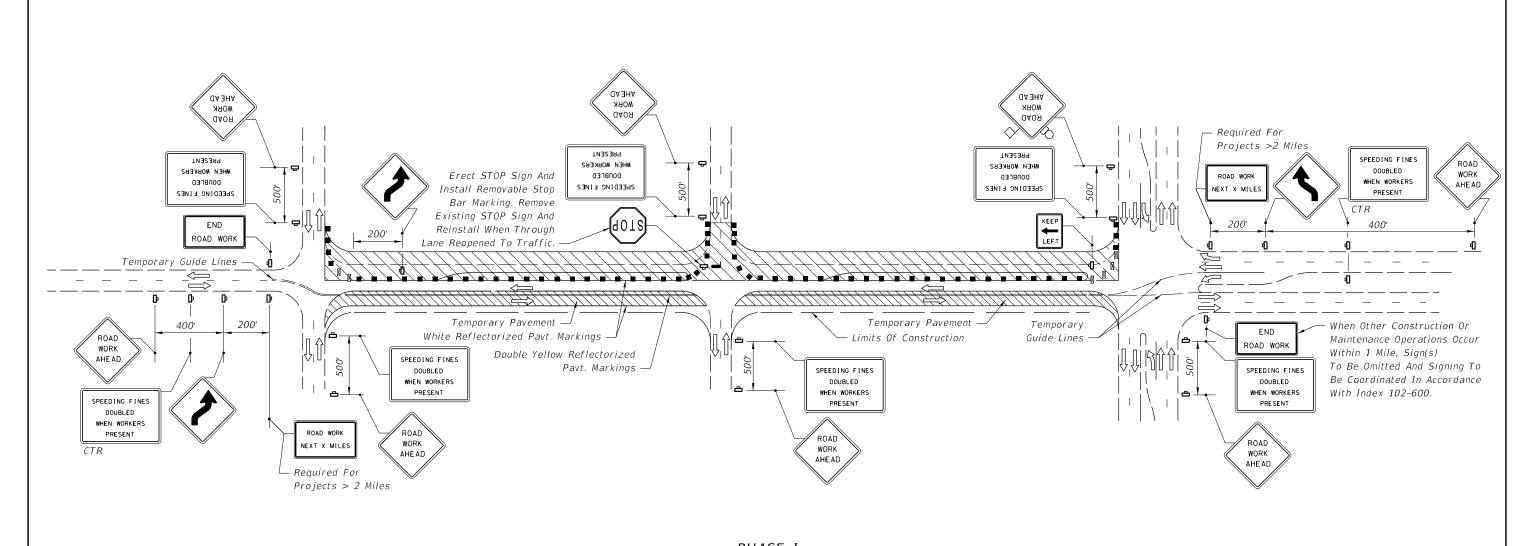
SHEET 1 of 2







102-640



## PHASE I

- 1. Maintain two-lane two-way traffic along existing facility. Install construction signing.
- 2. Remark existing pavement to facilitate temporary pavement construction. For lane width requirements see Index 102-600.
- 3. Construct temporary pavement of sufficient width to accommodate two-lane two-way traffic on the temporary pavement and a portion of the existing pavement during Phase I roadway construction. When two-lane two-way traffic can not be maintained during temporary pavement construction one-lane operations shall be maintained in accordance with Index 102-605. Channelizing devices shall be in conformance with 'Drop-Offs in Work Zones' of Index 102-600.
- 4. Mark the pavement in accordance with the Phase I diagram. Reroute through traffic to the temporary pavement and a portion of the existing pavement. For lane width requirements see Index 102-600.
- 5. Construct two lanes of the proposed roadway, excluding the friction course. Side street traffic to be maintained. Through and cross traffic to be controlled in accordance with Indexes 102-604, 102-605, and 102-615. Barricading shall be in conformance with 'Drop-Offs in Work Zones', Index 102-600. When work extends through an intersection, temporarily reroute the cross traffic to other cross streets. When rerouting is not possible, provide one-lane access (minimum) for two-lane two-way cross streets and one-lane access (minimum) each direction for four-lane two-way cross streets, in accordance with Indexes 102-604, 102-605, and 102-615.

Phase III Construction

Phase I Construction

Phase II Construction

*LEGEND* 

See Sheet 3 for General Notes.

**REVISION** 11/01/17

**SYMBOLS** 

Type III Barricade

Work Zone Sign

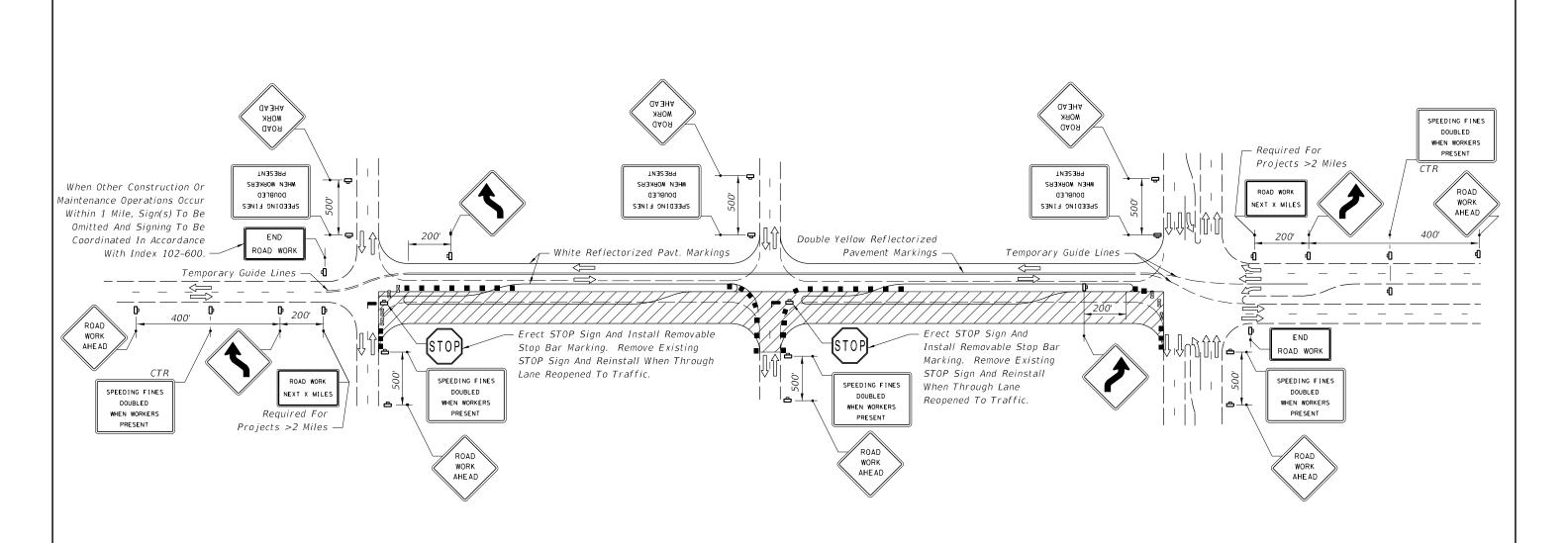
Stop Bar

Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic

DESCRIPTION:

FY 2019-20 STANDARD PLANS



#### PHASE II

- 1. Sign and mark Phase I pavement in accordance with the Phase II diagram. For lane width requirements see Index 102-600.
- 2. Reroute through traffic to Phase I pavement.
- 3. Complete all Phase II construction, including the friction course. Side street traffic to be maintained. Through and cross traffic to be controlled in accordance with Indexes 102-604, 102-605, and 102-615. Channelizing devices shall be in conformance with 'Drop-Offs in Work Zones' of Index 102-600. When work extends through an intersection, temporarily reroute cross traffic to other cross streets. When rerouting is not possible, provide one-lane access (minimum) for two-lane two-way cross streets and one-lane access (minimum) each direction for four-lane two-way cross streets, in accordance with Indexes 102-604, 102-605, and 102-615.

**LEGEND** 

Phase I Construction

Phase II Construction

Phase III Construction

See Sheet 3 for General Notes.

**REVISION** 11/01/17

**SYMBOLS** 

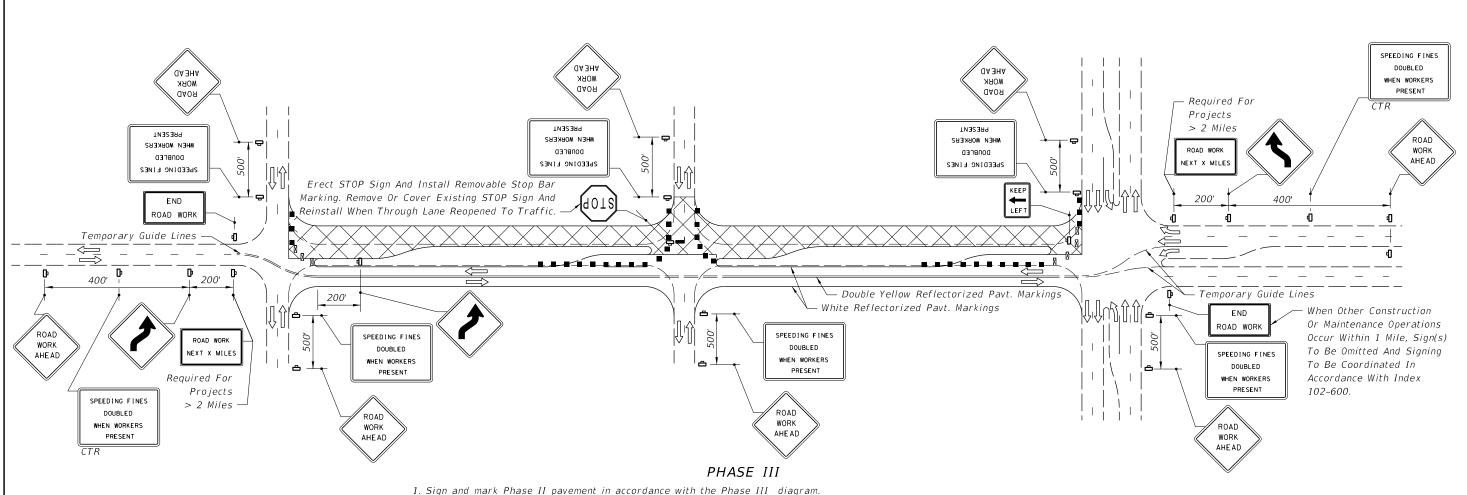
Type III Barricade

DESCRIPTION:

Work Zone Sign

Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic



- 2. Reroute through traffic to Phase II pavement.
- 3. Construct friction course over Phase I pavement. Side street traffic to be maintained. Through and cross traffic to be controlled in accordance with Index 102-604, 102-605, or 102-615. When work extends through an intersection, temporarily reroute cross traffic to other cross streets. When rerouting is not possible, provide one-lane access (minimum) for two-lane two-way cross streets and one-lane across (minimum) each direction for four-lane two-way cross streets.

#### GENERAL NOTES

- 1. All signing, pavement marking, and barricades necessary for maintenance of traffic shall conform to Index 102-600.
- 2. Lane widths for maintenance of two-way traffic should desirably be equal to lane widths of the existing facility, but lanes shall not be less than 10' in width. When one-lane one-way operations are necessary, a minimum width of 12' should be maintained and traffic controlled in accordance with Index 102-604, 102-605, or 102-615.
- 3. At signalized intersections, signals shall be directed or relocated as required to the center of relocated lanes.
- 4. For reflectorized raised pavement marker application, see Indexes 102-600 and 706-001.
- 5. Additional barricades, signing, lighting or other traffic controls for limited work areas shall be provided in accordance with other applicable TCZ Indexes as conditions warrant in each phase.
- 6. Provisions approved by the Engineer shall be made for the removal of storm water from the roadway(s) during construction.

## **LEGEND**

Phase I Construction

Phase II Construction

KXXX Phase III Construction

7. For general TCZ requirements and additional information, refer to Index 102-600.

**REVISION** 11/01/17 SYMBOLS

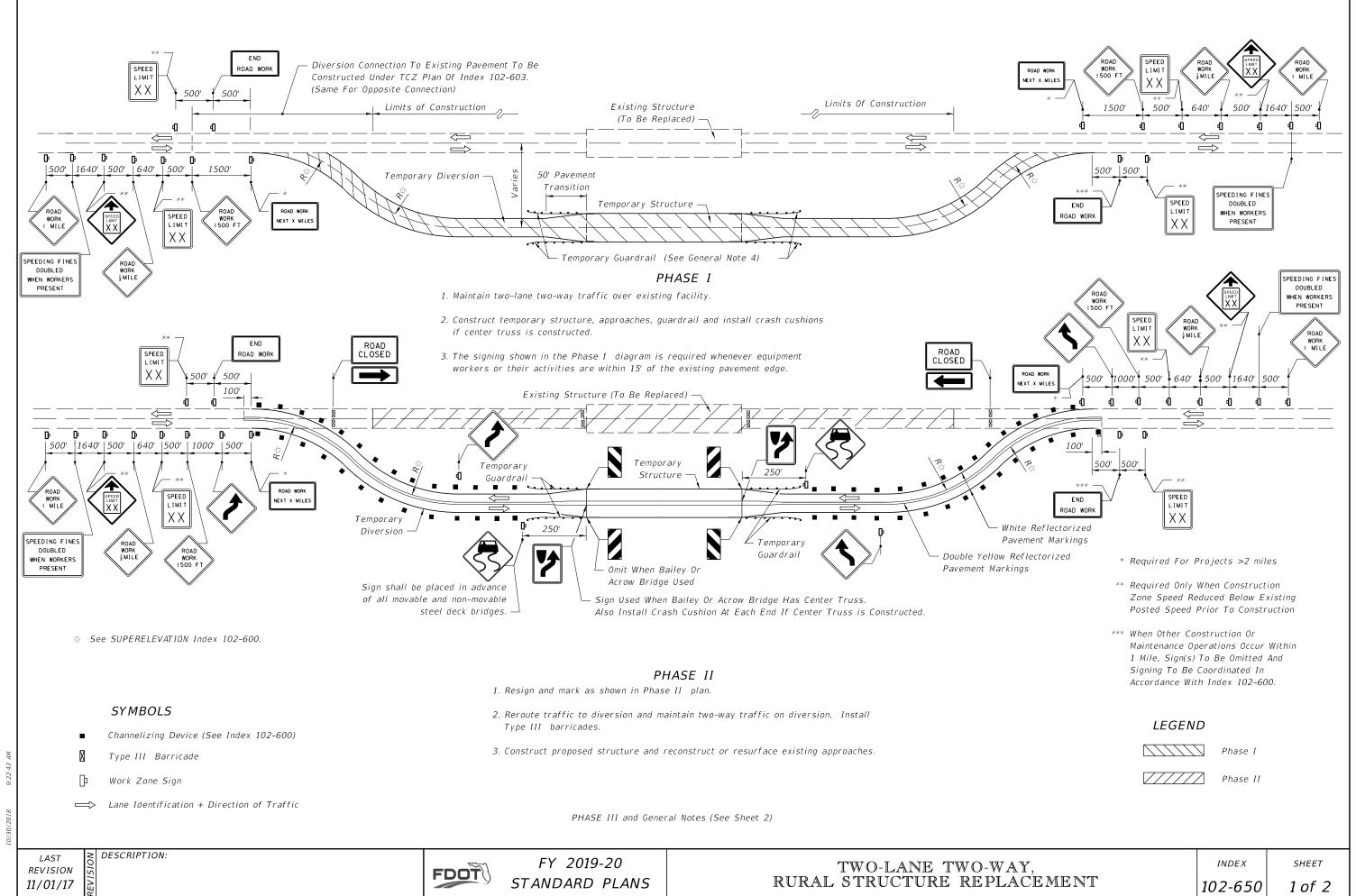
Type III Barricade

Work Zone Sign

DESCRIPTION:

Channelizing Device (See Index 102-600)

Lane Identification + Direction of Traffic



#### PHASE III

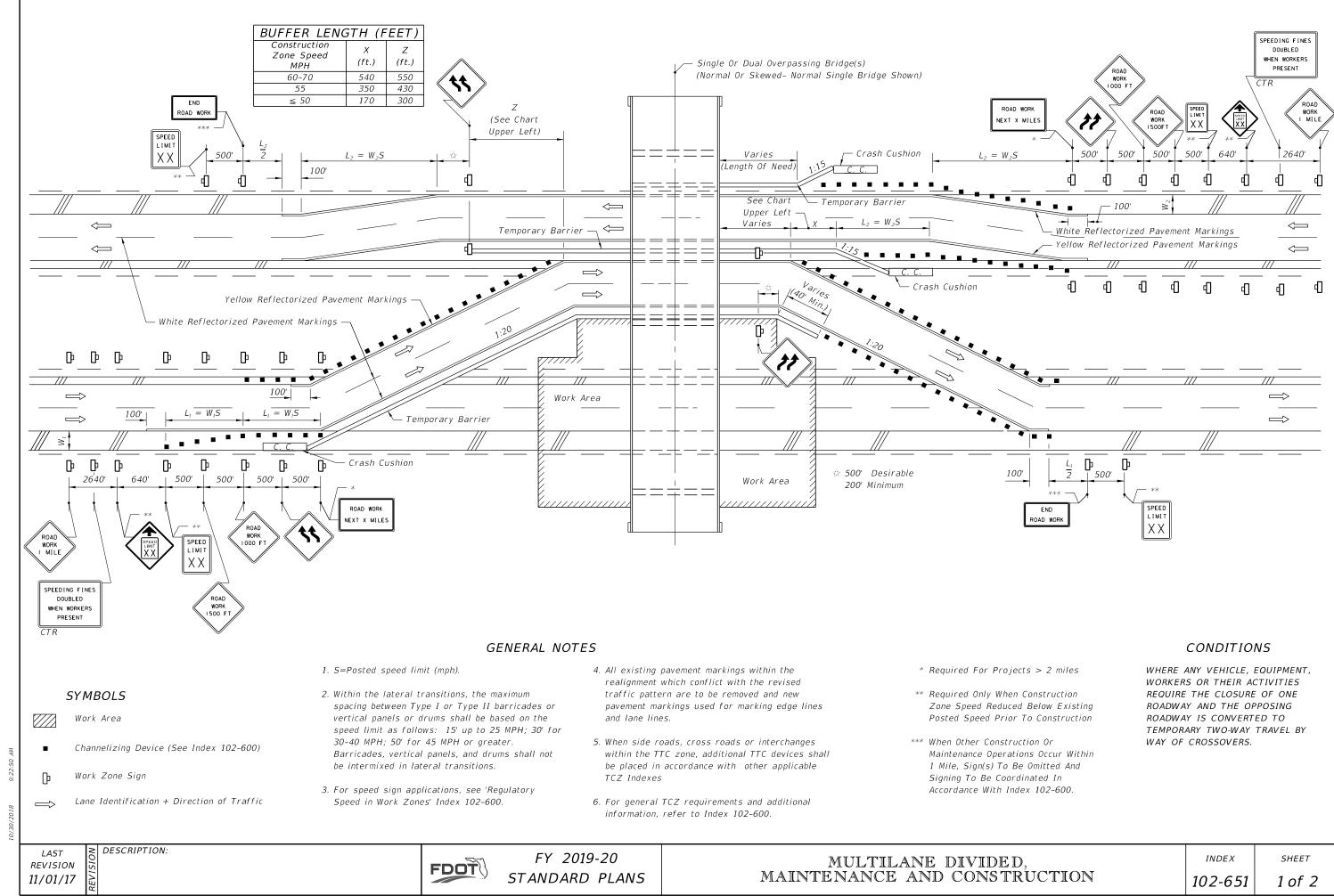
- 1. Reroute traffic to final alignment and maintain two-way traffic.
- 2. Remove all temporary construction items.

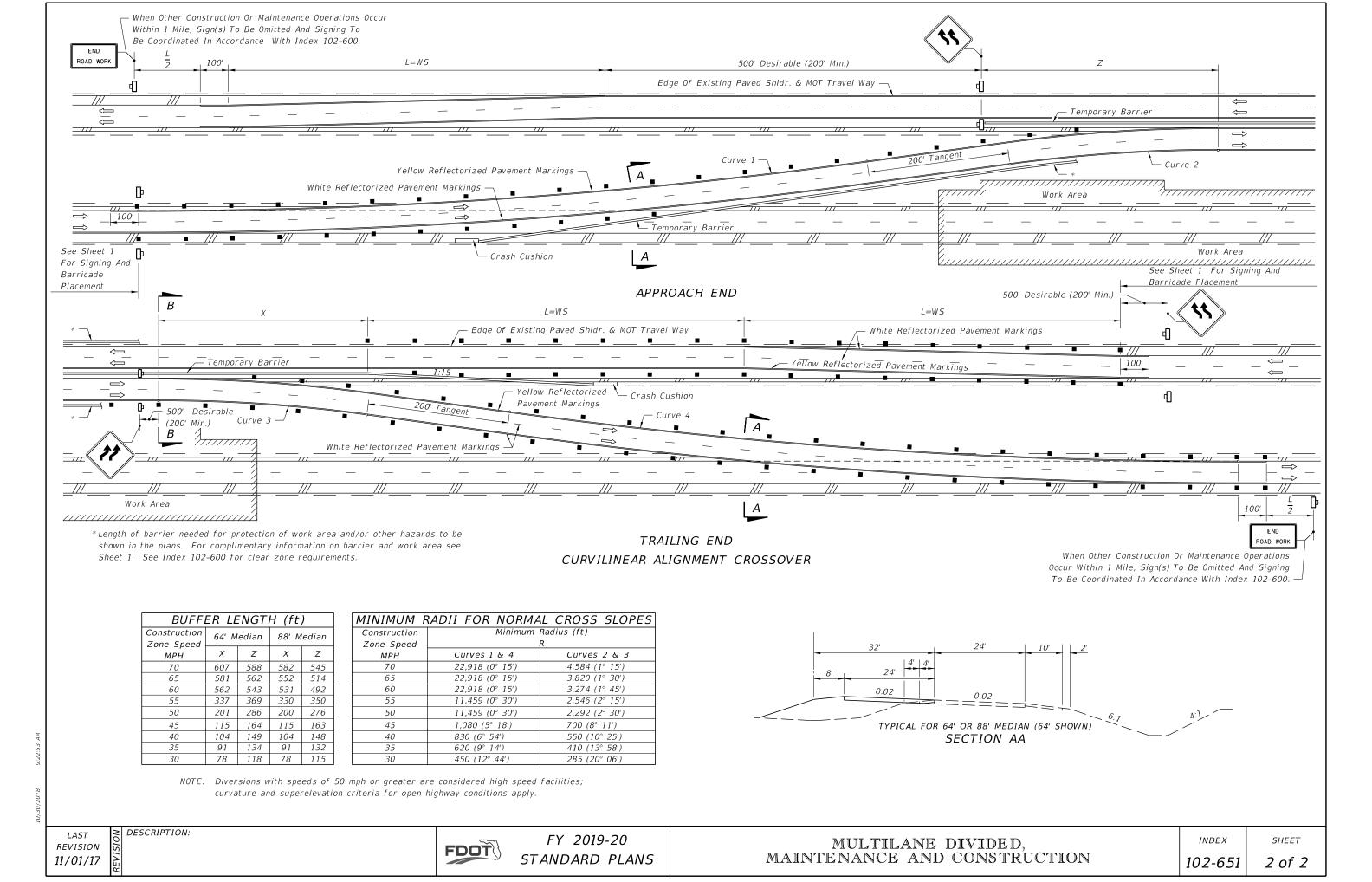
#### GENERAL NOTES

- 1. All signing, pavement marking, and barricades necessary for maintenance of traffic shall conform to Index 102-600.
- 2. For speed sign applications, see Index 120-600.
- 3. For lane width requirements see Index 102-600. When one-way one-lane operations are necessary, a minimum width of 12' shall be maintained and traffic controlled in accordance with Index 102-603, 102-606, or 102-607. Minimum width for the diversion shoulders is 6'.
- 4. Method of attaching temporary guardrail to the diversion structure to be approved by the Engineer. Cost of temporary guardrail systems, including end anchorage assemblies, transitions and attachment to temporary structures, are to be included in the contract unit price for Guardrail (Temporary) LF.
- 5. Provisions approved by the Engineer shall be made for the removal of storm water from the roadway(s) during construction.
- 6. Only temporary crash cushions approved by the Department shall be used unless specified devices called for in the plans.
- 7. Where the temporary structure is not required, the diversion may be constructed in accordance with Index 102-608, unless otherwise stipulated in the plans.
- 8. For reflective raised pavement marker application, see Indexes 102-600 and 706-001.
- 9. For general TCZ requirements and additional information, refer to Index 102-600.

MA 71.55.0

DESCRIPTION:

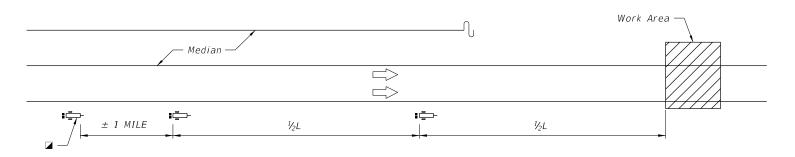




#### TRAFFIC PACING GUIDE

Traffic pacing is a traffic control technique to slow but not stop traffic to facilitate short duration work operations without an elaborate and difficult detour or diversion. Traffic Control Officers pace or slow the traffic to a speed that provides approximately 20–30 minutes to perform the work operation. The Department has frequently used this technique for setting bridge beams, overhead sign structures and replacing overhead sign panels.

# CHANGEABLE MESSAGE SIGNS (Typical Placement and Messages)



L = Length of Traffic Pacing Operation

## CHANGEABLE MESSAGE SIGN MESSAGE (MAINLINE AND RAMPS)

## Symbols

■ Channelizing Device (See Index 102-600)

Marked Police Vehicle with Flashing Blue Lights

PCMS, Portable Changeable Message Sign

To be placed the day of pacing operation

□> Lane Identification and Direction of Traffic

ONE WEEK PRIOR TO PACING OPERATION

DURING DAY
OF PACING OPERATION

DURING PACING OPERATION

EXPECT	MMM
DELAYS	DD-DD
ON	X AM - X AM
ROAD	EXPECT
WORK	PERIODIC
TONIGHT	DELAYS
SLOW	BE
TRAFFIC	PREPARED

TO STOP

#### NOTICE

This Index represents the minimum requirements for traffic pacing operations on the State Highway System.

Develop a site specific traffic control plan for each pacing operation location.

#### TRAFFIC PACING GENERAL NOTES

- 1. Install ROAD CLOSED (W20-3) signs approximately 1000' prior to the work area. These signs shall remain covered until the pacing operation begins and covered when the pacing operation has ended.
- 2. Prior to requesting that the traffic control officer supervisor initiate the pacing operation, the contractor shall ensure that the necessary equipment is properly positioned (off the roadway) for the construction activity requiring the traffic pacing operation.
- 3. Truck mounted attenuator(s) with changeable message sign(s) are required to protect workers and/or equipment positioned in a travel lane(s) at the work area during the pacing operation from an errant vehicle. If no workers and/or equipment are positioned in a travel lane(s) at the work area, truck mounted attenuator(s) are not required.
- 4. A traffic control officer supervisor shall be stationed at the work area continuously throughout the pacing operation to insure radio communications between the contractor and/or the project administrator, and all the police vehicles involved in the pacing operation.
- 5. When more than one pacing operation is required in one work period the contractor shall allow sufficient time between pacing operations to permit traffic to return to normal speeds and flow. Additional time may be required between pacing operations to allow traffic to resume normal speeds and flow upstream of the work area as determined by the project administrator or traffic control officer supervisor.
- 6. For work durations of less than five minutes, coordinate with traffic control officer to provide resources necessary for pacing traffic. Portable changeable message signs, truck-mounted attenuators, ROAD CLOSED signs, and site specific traffic control plans are not required for such operations. Use traffic pacing distance values from the five minute column of the table on Sheet 3.

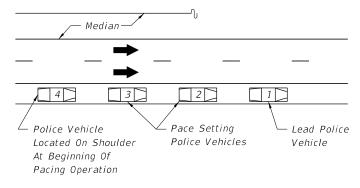
#### TRAFFIC CONTROL PLANS OR TECHNICAL SPECIFICATION

- 1. The specific activities and locations, along with allowable times of day and days of the week, when pacing will be allowed should be clearly detailed in the traffic control plans or technical specification. If there are specific holiday or special event dates that, due to anticipated traffic congestion, pacing operations should not be allowed, these dates should also be spelled out in plans or specifications. When detailing the specific activities and locations of pacing activities, identify the minimum number of traffic control officers needed for each function and location of the pacing operation. If there are certain work activities that need to be completed prior to the contractor starting the work anticipated during the pacing operation, the activities should be clearly detailed in the plans or technical specification.
- 2. When developing a pacing plan, failsafe "stop points" should be identified for those work operations in which a construction problem could create a condition that could not be immediately cleared. A failsafe stop point is the last safe egress from the highway facility prior to traffic coming upon the work that is being completed during the operation. In the unlikely event that the work is not completed during the time estimated for the pacing, the plans or specification should direct the pacing to not proceed past the failsafe stop point until the highway is cleared. In the event of major construction problem that cannot be immediately cleared, traffic can then be diverted off the facility.
- 3. The traffic control plans or technical specification should require the contractor to submit a pacing plan in advance of the operation. The pacing plan should outline the contractors expected equipment and personnel, outline the operation, and include a contingency plan should any of the contractor's critical equipment break down. If the project includes a damage recovery clause, the traffic control plan or technical specification should be clear that the damage recovery applies to the pacing operation as well.
- 4. Changeable message signs shall be displayed one week prior to work using messages described in the traffic pacing plan. The number and location of changeable message signs shall be called out in the traffic control plans.

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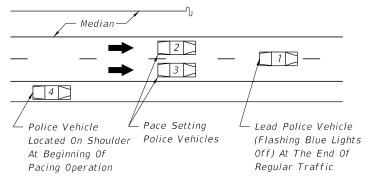
AHEAD

## MAINLINE PACING DETAILS (1 DIRECTION OF FOUR LANE ROADWAY EXAMPLE)



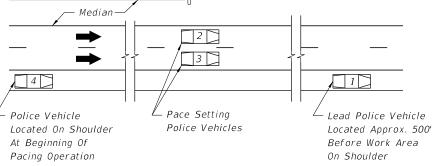
#### STAGE ONE

1. Four police vehicles located upstream of the work area at the beginning location of the traffic pacing operation with flashing blue lights off.



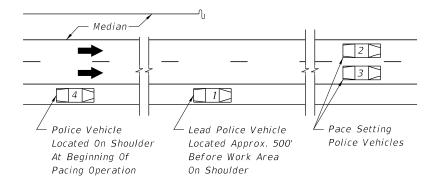
#### STAGE TWO

1. Once the police vehicles are in place and the traffic control officer supervisor at the work area notifies all officers to begin the traffic pacing operation, the last three police vehicles shall turn on their flashing blue lights. The first three police vehicles shall enter the travel lanes with the second and third police vehicles immediately forming a side by side "pacing operation" of all lanes behind the lead police vehicle (flashing blue lights off).



#### STAGE THREE

- 1. The two pace setting police vehicles shall begin to slow to the pacing speed (20 mph is preferred, 10 mph minimum), for the duration of the traffic pacing operation.
- 2. The lead police vehicle (flashing blue lights off) shall match the speed of the last vehicles ahead of the pacing vehicles and continue following traffic until a point approximately 500' in advance of the work area. The lead police vehicle shall then come to a complete stop on the right shoulder and turn on its flashing blue lights. If required, crash truck(s) with rear mounted impact attenuator(s) and changeable message sign(s) shall move into the travel lanes approximately 200 ft. upstream of the work area with the impact attenuators down and operating once traffic has cleared the work area.

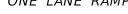


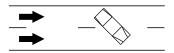
#### STAGE FOUR

- 1. When the pace setting police vehicles are within approximately two miles of the work area they shall notify the onsite traffic control officer supervisor who will immediately inform the contractors on site supervisor of their location. Once the contractors on site supervisor has been notified of the pacing vehicles location, the contractor shall begin to clear the travel lanes of all equipment and debris in order to reopen all travel lanes.
- 2. In case of emergency the pace setting police vehicles shall come to a complete stop once they reach the lead police vehicle. If no emergency is encountered, the crash truck(s) shall be moved from the travel lanes and the two pace setting police vehicles shall clear the work area and immediately move to the right shoulder or an area designated by the traffic control officer supervisor and turn off the flashing blue lights. Once the two pace setting police vehicles pass the work area, the traffic control officer supervisor shall instruct the lead and last police vehicles to turn off their flashing blue lights.

## RAMP PACING DETAILS







TWO LANE RAMP

#### RAMP CLOSURE DETAIL

- 1. Once notified by the on site traffic control officer supervisor to begin the traffic pacing operation each police vehicle at the indicated ramp shall turn their flashing blue lights on and position the vehicle across the ramp lane(s) to close ramp access.
- 2. Once the pacing operation passes the closed on ramp the police vehicle on the ramp shall turn off the flashing blue lights and move from the ramp lane(s) to allow traffic to enter the mainline pacing operation.

#### GENERAL NOTES

1. Each Traffic Control Officer shall have a marked vehicle with flashing blue lights, for the pacing operation. The location and number of officers at each location will be as follows:

No. Of Traffic Control Officers With Vehicles	Function	Location
1 min.	Supervisor	Work Area
1 Lead Vehicle	Varies	Mobile operation
1 for each travel lane	Pacing Operation	Mobile operation beginning x miles upstream and terminating at the work area
1 Stationed at the Beginning of Pacing Operation	Advanced Warning to Motorist	Stationed at the Beginning of Pacing Operation
1 for each entrance ramp	Entrance Ramp Roadblocks	One at each of the entrance ramps upstream of the work area

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LAST REVISION 11/01/17

DESCRIPTION:

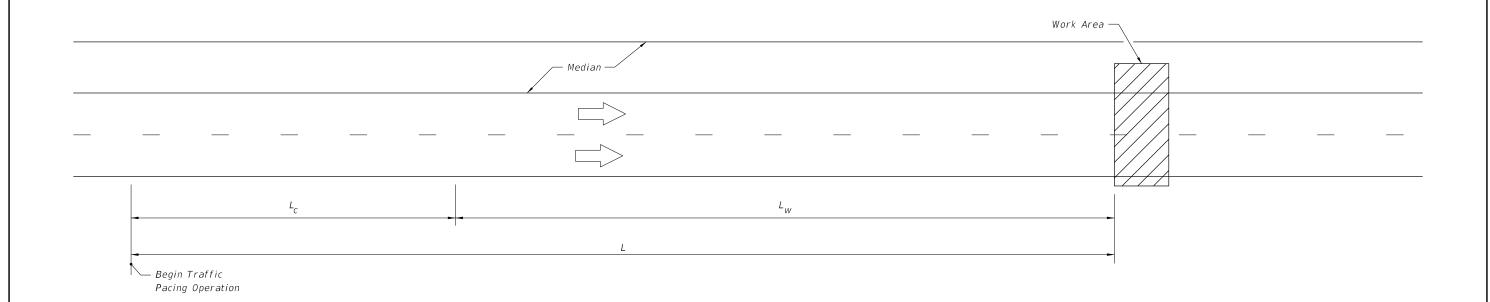
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TRAFFIC PACING

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#### DESIGN CONSIDERATIONS:

The design shall evaluate the actual distance required for the pacing operation based on site specific features such as: roadway geometrics, pacing speeds, regulatory speeds, interchange spacing, work duration, availability of traffic control officers, traffic volumes and maximum queue length.

The starting point of a traffic pacing operation must consider the following factors: the speed of the pacing vehicles, the location of entrance ramps, horizontal and vertical alignment of the

In some instances, it may be necessary to close a lane at the work site to position a crane(s) and the materials to be lifted.

All material to be installed shall be on-site before the traffic pacing operation begins.

It may be necessary to install temporary barrier walls to protect pre-positioned and assembled materials in the right of way.

The minimum speed allowed for a pacing operation is 10 mph with 20 mph the preferred speed.

The maximum allowed work duration is  $\frac{1}{2}$  hour (30 min).

The maximum practical pacing operation length is 10 miles.

 $S_r = Regulatory speed (mph)$ 

 $S_n = Pacing speed (mph)$ 

 $t_w = Work duration (min)$ 

L = Total pacing distance in miles

$$L = \frac{t_W}{60} S_p \left( \frac{S_p}{S_r - S_p} + 1 \right)$$
$$L = L_C + L_W$$

 $L_c$  = distance paced vehicles must travel before the vehicles at regulatory speed have cleared the work zone

$$L_{c} = \left(\frac{\frac{t_{w}}{60} \times S_{p}^{2}}{S_{r} - S_{p}}\right)$$

 $L_w$  = distance paced vehicles travel while work is performed

$$L_W = \left(\frac{t_W}{60} \times S_p\right)$$

 $F_{HV} = Heavy Vehicle Factor$ 

$$F_{HV} = 1 + \left(\frac{P_t}{100} \times 0.5\right)$$

 $P_t = \% Trucks$ 

## TRAFFIC PACING DISTANCES (L) miles

$S_p = 20;$	pcphpl	≤	1,750

s _r			t _w (	min)		
-r	5	10	15	20	25	30
70	2.3	4.7	7.0	9.3	*	*
65	2.4	4.8	7.2	9.6	*	*
60	2.5	5.0	7.5	10.0	*	*
55	2.6	5.2	7.9	*	*	*
50	2.8	5.6	8.3	*	*	*

 st  Calculation required, for additional guidance see FD0T Design Manual 242.

#### NOTES FOR TABLE:

 $\mathbf{t}_{\mathbf{W}}$  is the total time allowed for work activity in minutes. This time starts just after the last vehicle traveling at the pre-pacing regulatory speed clears the work area and ends just as the pacing operation reaches the work area.  $t_W$  must include the time required to clear the roadway of equipment, materials, and personnel.

Demand volume may not exceed 1,750 pcphpl (passenger cars per hour per lane) without a site specific design. Traffic counts can be obtained from the Office of Planning, or you may need to collect traffic counts. Hourly directional traffic volumes must be converted to pcphpl using the following:

$$pcphpl = \left(\frac{Hourly\ Directional\ Volume}{\#\ Lanes\ (each\ direction)}\right) x\ Heavy\ Vehicle\ Factor$$

DESCRIPTION:

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TRAFFIC PACING

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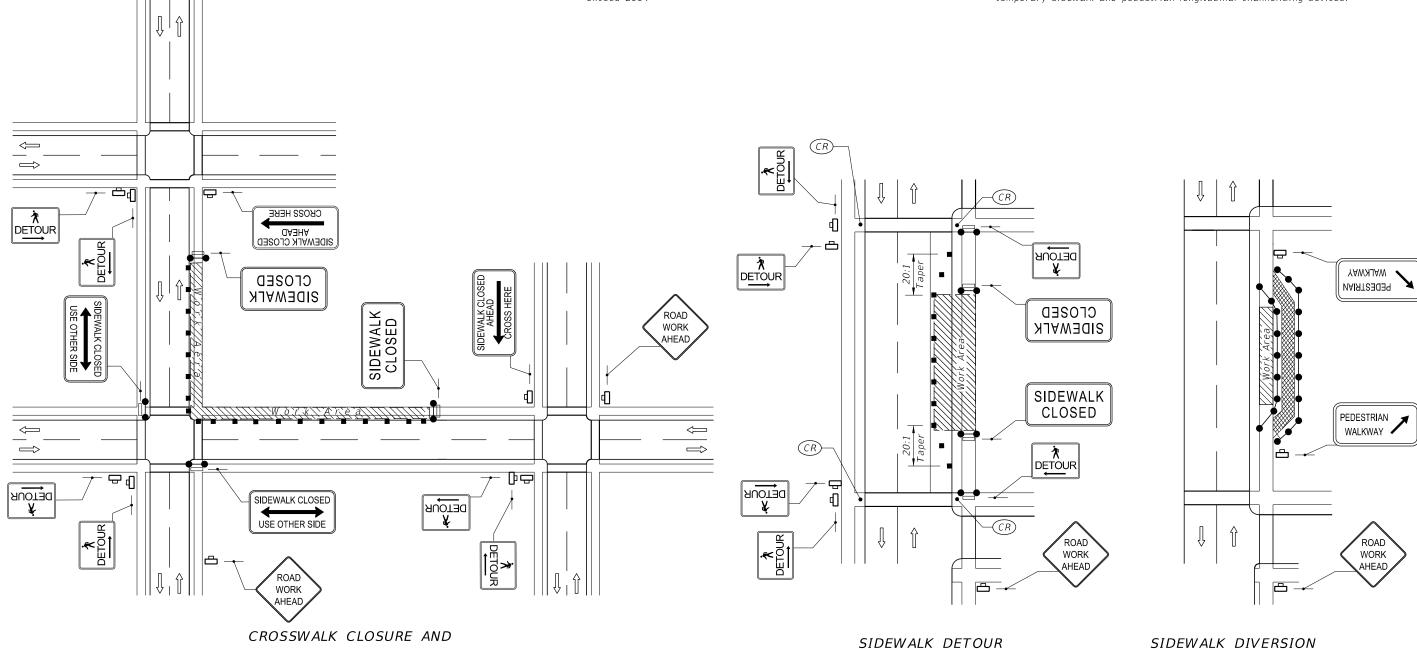
#### **SYMBOLS**

- Work Area
- Channelizing Device
- Work Zone Sign
- - Required Locations For Either Temporary Or Permanent Curb Ramps.
- Lane Identification + Direction of Traffic
- Pedestrian Longitudinal Channelizing Device (LCD) with Mounted Work Zone Sign or separate Work Zone Sign
- Pedestrian Longitudinal Channelizing Device (LCD)
- Temporary Sidewalk

#### **GENERAL NOTES:**

- 1. When encroaching work requires a sidewalk closure for 60 minutes or greater, provide an alternate pedestrian route.
- 2. For spacing of vehicular Channelizing Devices, see applicable vehicular temporary traffic control Indexes.
- 3. Cover or deactivate pedestrian traffic signal display(s) controlling closed crosswalks.
- 4. For post mounted signs located near or adjacent to a sidewalk, maintain a minimum 7' clearance from the bottom of the sign panel to the surface of the sidewalk.
- 5. Provide a 5' wide temporary walkway, except where space restrictions warrant a minimum width of 4'. Provide a 5' x 5' passing space for temporary walkways less than 5' in width at intervals not to exceed 200'.

- 6. Provide a cross-slope with a maximum value of 0.02 for all temporary walkways.
- 7. Maintain temporary walkway surfaces and ramps that are stable, firm, slip-resistant, and free of any obstructions or hazards such as holes, debris, mud, construction equipment, and stored material.
- 8. Remove temporary walkways immediately after reopening of the sidewalk, unless otherwise noted in the plans.
- 9. Meet the requirements of Index 522-002 for temporary curb ramps.
- 10. Place pedestrian longitudinal channelizing device(s) across the full width of the closed sidewalk. For temporary walkways, similar to the Sidewalk Diversion, place LCDs to delineate both sides of the temporary walkway.
- 11. For sidewalk diversions, ensure that there is sufficient R/W for placement of temporary sidewalk and pedestrian longitudinal channelizing devices.



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DESCRIPTION:

PEDESTRIAN DETOUR

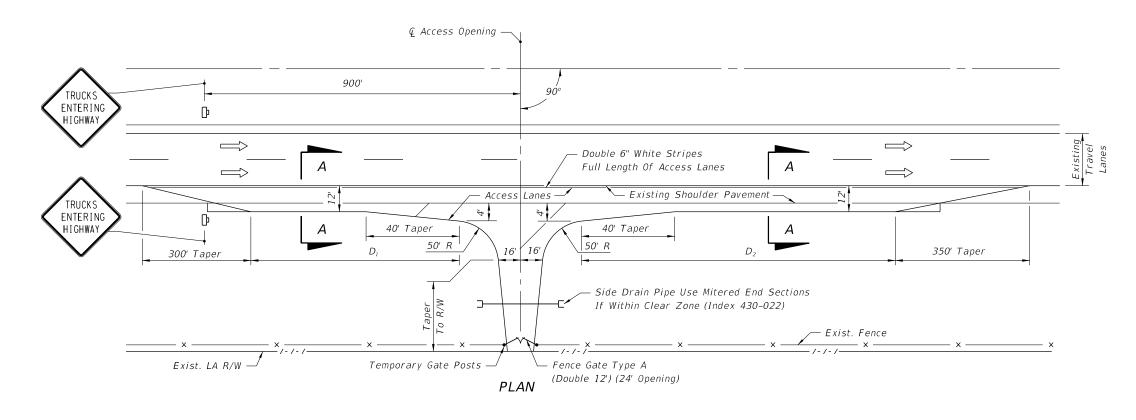
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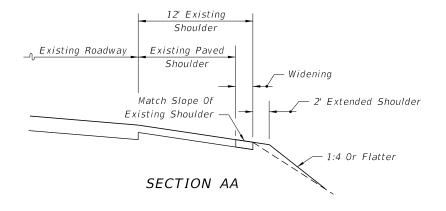
PEDESTRIAN CONTROL FOR CLOSURE OF SIDEWALKS

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LENGTH OF ACCESS LANES (Ft.)			
ELINGITI OF ACCESS LANES (I.I.)			
Grade	$D_1$	$D_2$	
2% or less	590	1540	
3 to 4% Upgrade	530	2310	
3 to 4% Downgrade	710	925	



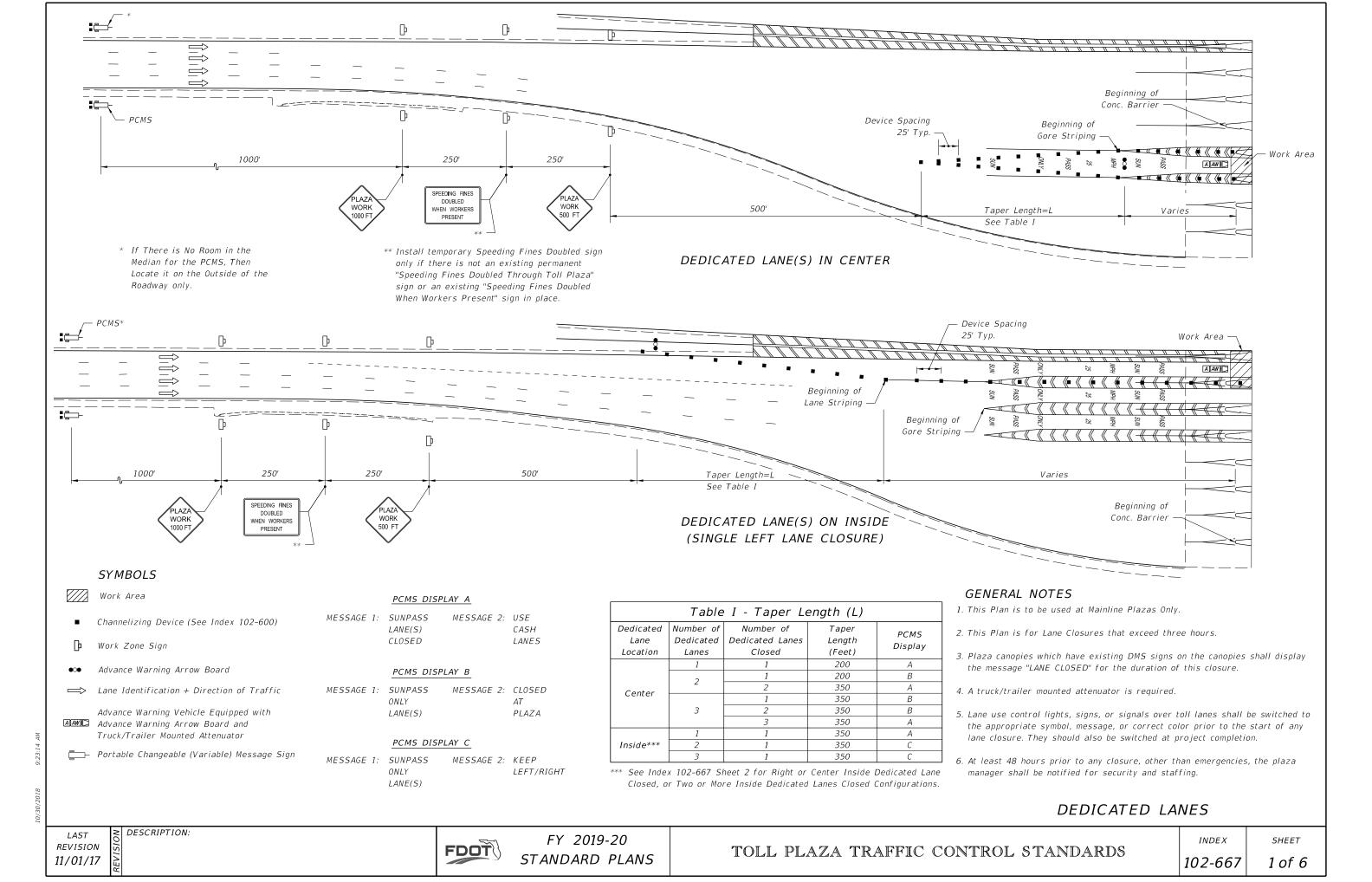
- 1. Access openings across limited access right of way and use of this Index are prohibited unless specifically permitted in the Contract Plans or Special Provisions. When permitted in the Contract Plans or Special Provisions and prior to construction of any opening, the Contractor must submit, in writing, a request identifying specific locations for approval by the Engineer.
- 2. No more than two (2) access openings will be allowed on each project.
- 3. Access openings shall be located only in areas having adequate sight distance and shall not be located within 1.5 miles of interchanges nor within 2000 ft. of acceleration-deceleration lanes at rest areas, other access openings or other highway service areas.
- 4. Access openings shall not be constructed directly opposite temporary median crossovers nor within 2000 ft. of temporary median crossovers.
- 5. Access openings shall be within the project limits and shall not be used for transporting materials to or from any other project. The acceleration-deceleration surfaces shall be paved. RAP material is acceptable for driveway surfacing.
- 6. Any Motorist Aid Call Boxes affected by the temporary access openings shall be relocated outside the limits of access lanes and remain in use during construction. Upon removal of access lanes, call boxes shall be returned to their previous location. Temporary relocation and restoration of call boxes shall be at the contractor's expense.

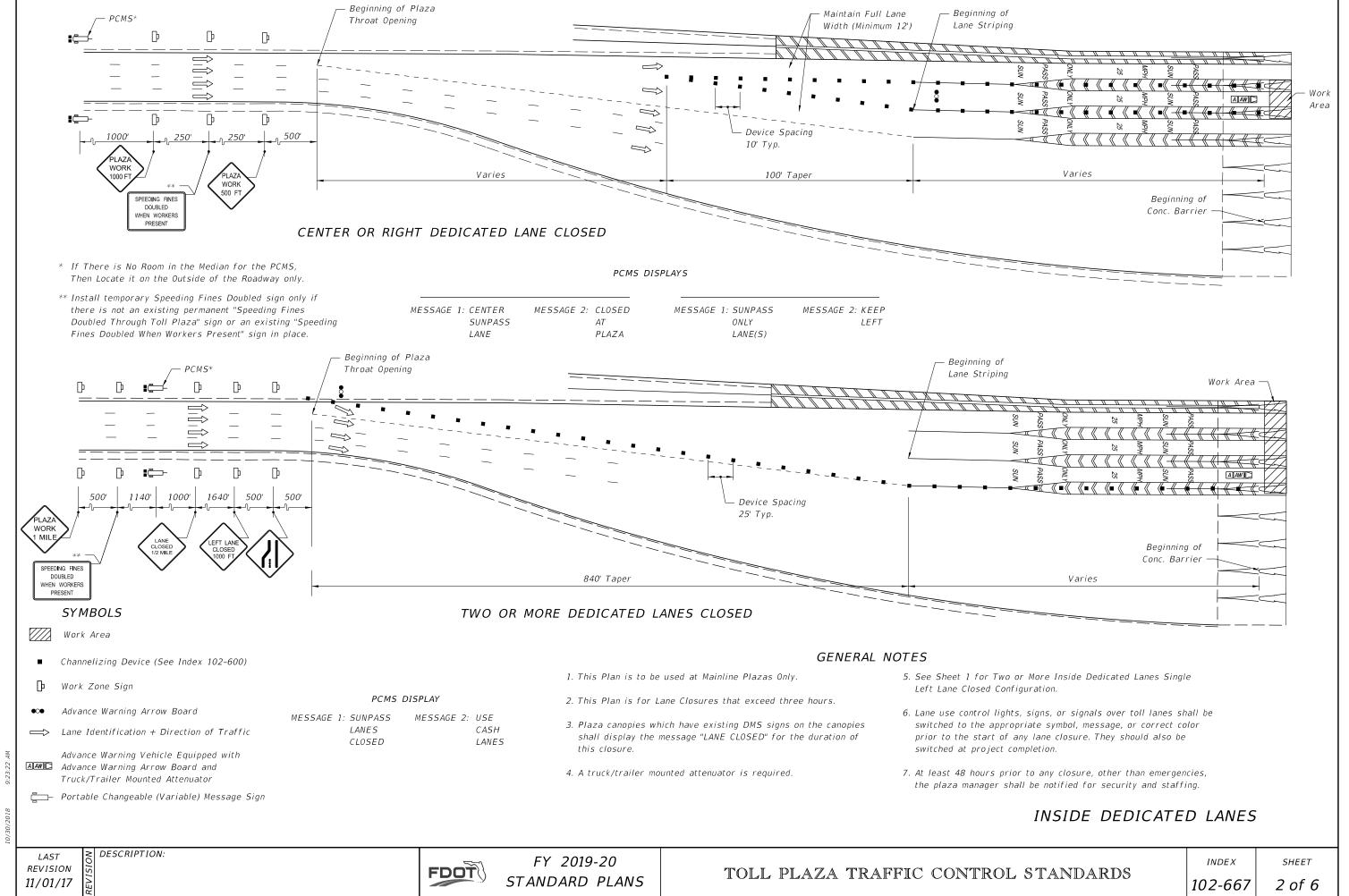
- GENERAL NOTES
  - 7. Access openings in the limited access fence shall have gates which are to be locked during nonwork hours or periods when the access is not in active use.
  - 8. The contractor shall take all precautions necessary to insure against entrance by livestock or unauthorized persons or vehicles.
  - 9. The contractor shall not vary from the plan detail without approval of the Engineer.
  - 10. Gates shall be removed and access opening locations shall be restored to preconstruction condition immediately upon completion of activities utilizing the materials being transported through the openings whether or not the project is completed.
  - 11. Failure to comply with any provision of the access opening plan shall be cause for terminating use of all openings. Upon notification by the Engineer, the contractor shall cease hauling and begin restoration of affected areas. Under this condition expense of removal, restoration and of additional hauling distances shall be borne by the contractor.
  - 12. No guardrail or barrier wall will be removed for access openings.
  - 13. Construction and removal of the access and restoring the area to preconstruction condition shall be included in the cost of Maintenance Of Traffic, LS.

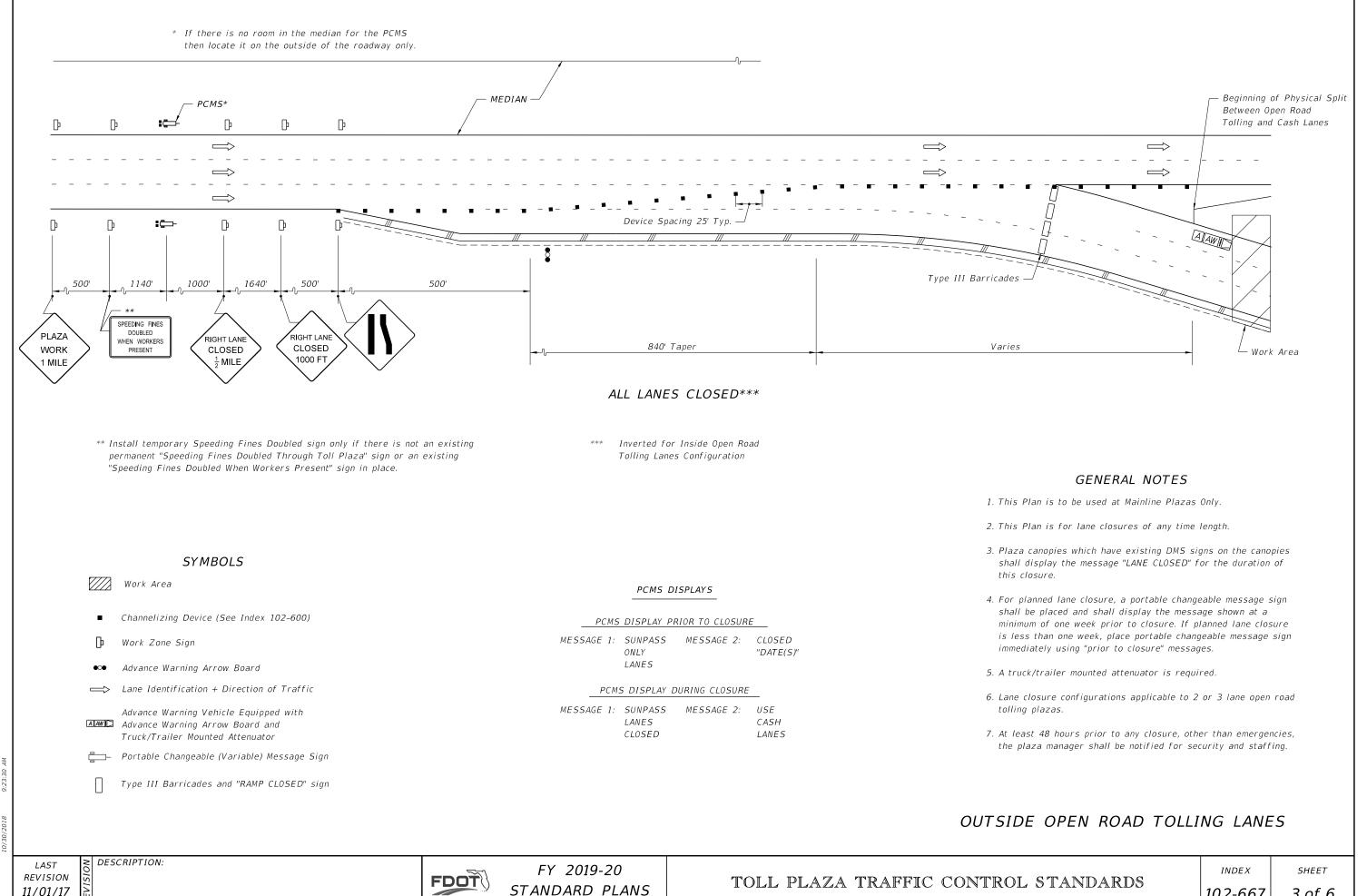
SYMBOLS

₩ork Zone Sign

LAST O DESCRIPTION:
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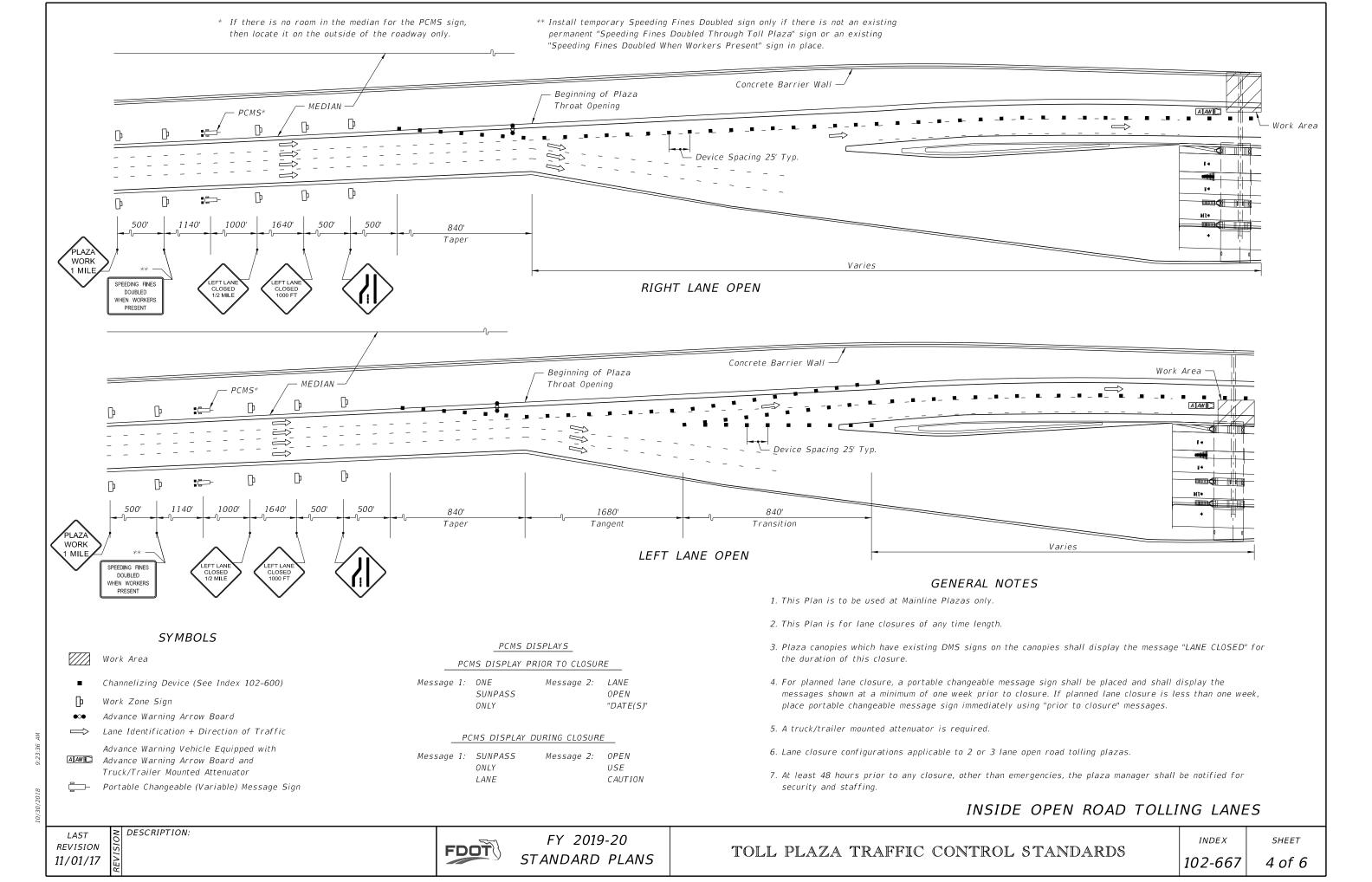


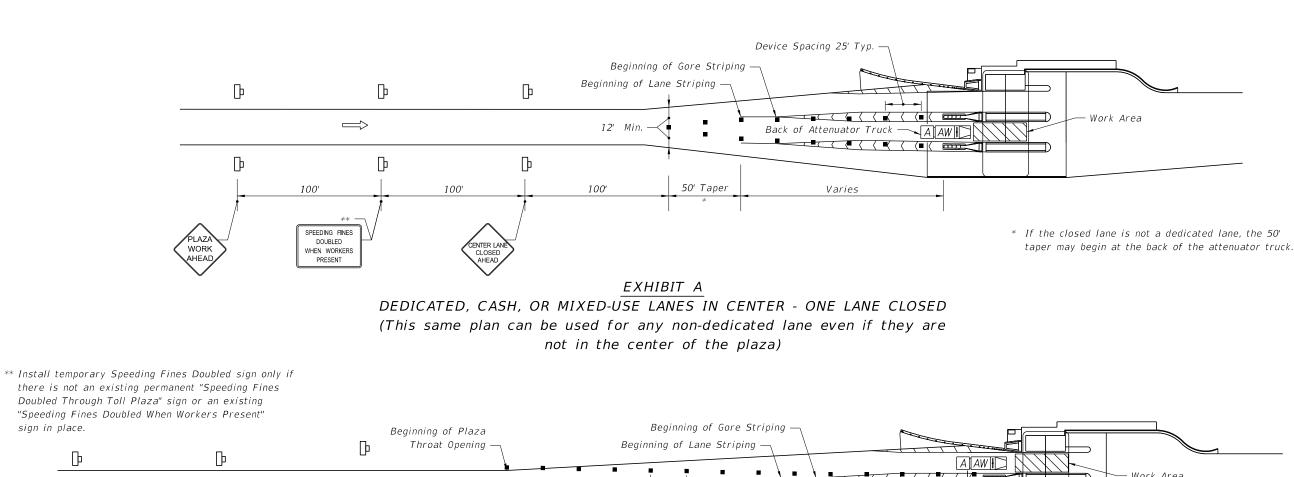


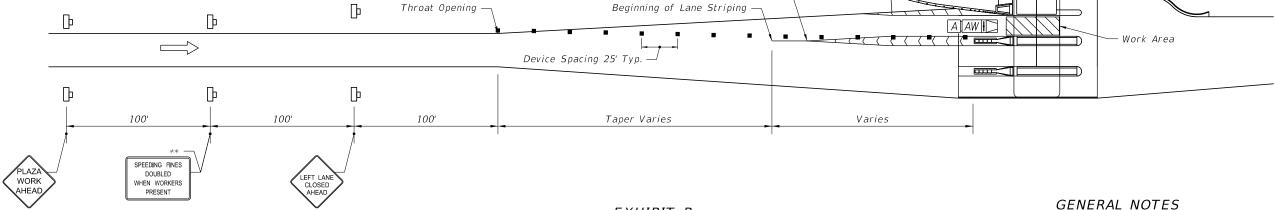
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## **SYMBOLS**

sign in place.

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

DESCRIPTION:

Advance Warning Vehicle Equipped with Advance Warning Arrow Board and Truck/Trailer Mounted Attenuator

## EXHIBIT B DEDICATED LANE INSIDE OR OUTSIDE - ONE LANE CLOSED

(Outside Lane Closure is a Mirror Image of this Exhibit)

- 1. This Plan is for lane closures that exceed three hours.
- 2. If the closed lane is a dedicated lane, Exhibit A shall be used at Ramp Plazas only. If the closed lane is a cash or mixed-use lane, Exhibit A may be used at Ramp or Mainline Plazas.
- 3. A truck/trailer mounted attenuator is required.
- 4. Exhibit B shall be used at Ramp Plazas only.
- 5. Lane use control lights, signs, or signals over toll lanes shall be switched to the appropriate symbol, message, or correct color prior to the start of any lane closure. They should also be switched at project completion.
- 6. At least 48 hours prior to any closure, other than emergencies, the plaza manager shall be notified for security and staffing.

## MAINLINE PLAZAS & RAMP PLAZAS

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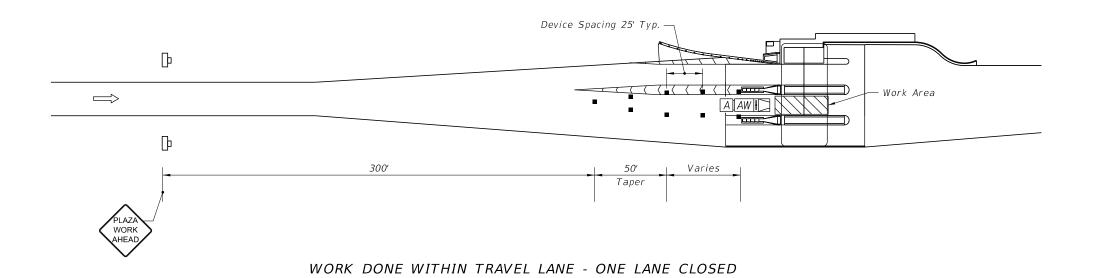
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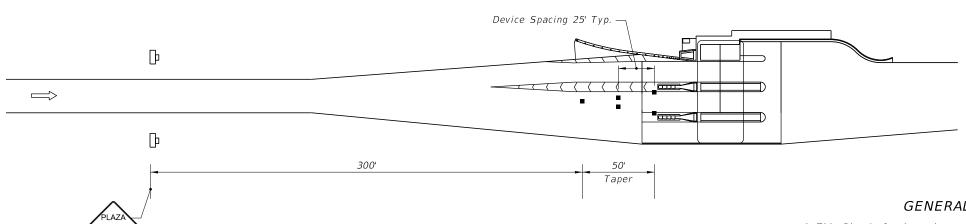
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TOLL PLAZA TRAFFIC CONTROL STANDARDS

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WORK NOT DONE WITHIN TRAVEL LANE - ONE LANE CLOSED

#### SYMBOLS

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

Lane Identification + Direction of Traffic

Advance Warning Vehicle Equipped with Advance Warning Arrow Board and Truck/Trailer Mounted Attenuator

#### GENERAL NOTES

- 1. This Plan is for lane closures that are three hours or less.
- 2. This Plan is to be used at Ramp or Mainline Plazas.
- 3. This plan can be used for any lane, with appropriate modifications, even if it is not in the center of the Plaza.
- 4. Lane use control lights, signs, or signals over toll lanes shall be switched to the appropriate symbol, message, or correct color prior to the start of any lane closure. They should also be switched at project completion.
- 5. At least 48 hours prior to any closure, other than emergencies, the plaza manager shall be notified for security and staffing.
- 6. A Truck/Trailer Mounted Attenuator is required for all aerial work operations (lift truck). For non-aerial operations, the Truck Mounted Attenuator or additional devices may be required by the Engineer based on the work being performed.

## SHORT-TERM CLOSURES

**REVISION** 11/01/17

DESCRIPTION:

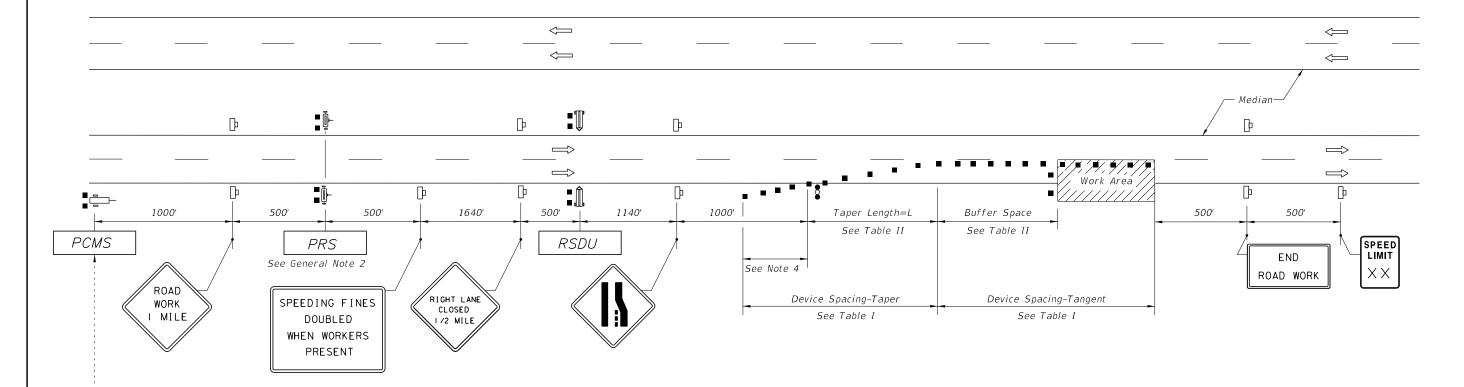


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#### TYPICAL PCMS DISPLAY

With speed reduction:

Message 1: WORKERS PRESENT AHEAD Message 2: SPEED REDUCED NEXT XMI

Without speed reduction:

Message 1: WORKERS PRESENT AHEAD

Message 2: NEXT X MILES

Table I				
Device Spacing				
	Max.	Distance	Between D	Pevices (ft.)
Posted	Cones or		Type I d	or Type II
Speed	Tubular		Barricades	or Vertical
(mph)	Markers		Panels (	or Drums
	Taper	Tangent	Taper	Tangent
55 to 70	25	50	50	100

Table II					
Buffer	Buffer Space and Taper Length				
Posted Speed	Buffer Space	,	Length I Transition)		
(mph)	Dist. (ft.)	L (ft.)	Notes (Merge)		
55	495	660			
60	570	720	L = WS		
65	645	780			
70	730	840			

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

For lateral transitions other than 12', use formula for L shown in the notes column.

L= Length of taper in feet

W= Width of lateral transition in feet

S= Posted speed limit (mph)

#### **SYMBOLS**

Work Area

Channelizing Device (See Index 102-600)

Work Zone Sign

Advance Warning Arrow Board

Lane Identification + Direction of Traffic

(1) PCMS= Portable Changeable(Variable) Message Sign

(2) PRS= Portable Regulatory Sign- Speed Limit When Flashing

(2) RSDU= Radar Speed Display Unit

DESCRIPTION:

#### **GENERAL NOTES:**

- 1. Use the MAS for lane closures of 5 day or more on multilane divided facilities with a posted speed of 55 MPH or greater when workers are present and not protected by a barrier.
- 2. For posted speeds of 65 MPH or greater, reduce Work Zone Speeds by 10 MPH. For posted speeds of 60 MPH, use a Work Zone Speed of 55 MPH.
- 3. Right lane closure shown, left lane closure similar using left lane signing.
- 4. Use shoulder taper in accordance with Index 102-612 for shoulder widths 8 feet or greater.
- 5. See Index 102-600 for general TCZ requirements and additional information.

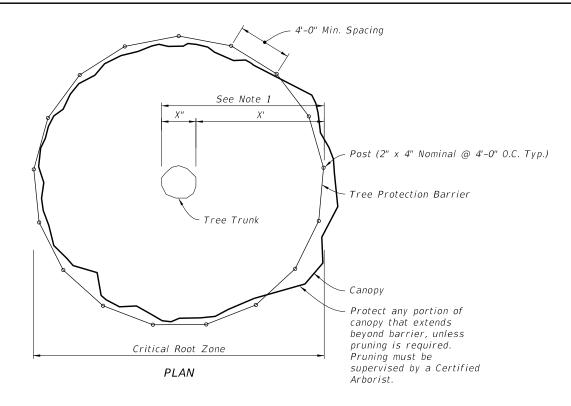
**REVISION** 11/01/17

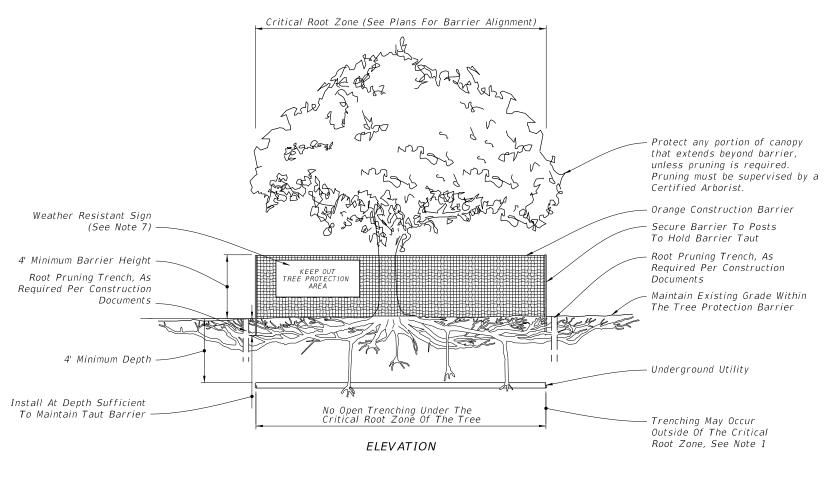
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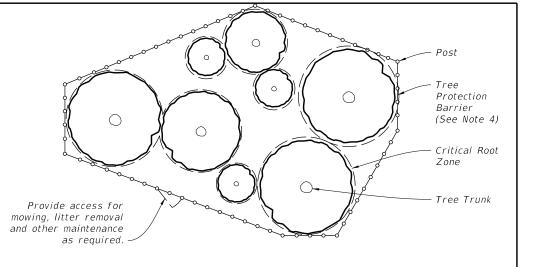
FY 2019-20 STANDARD PLANS

#### NOTES:

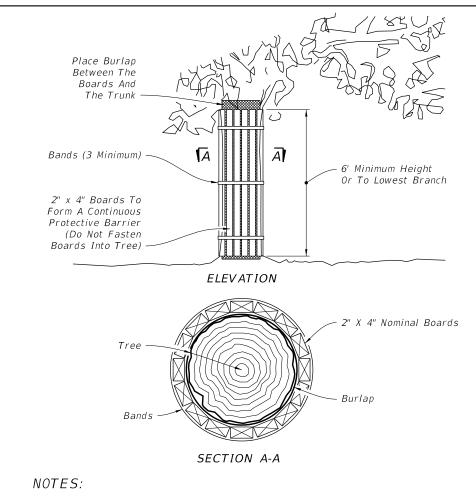
- 1. Critical Root Zone: Extends in all directions from trunk of tree to a distance equal to one foot per inch of trunk diameter at breast height.
- 2. Staging, storage, dumping, washing and operation of equipment is not permitted within the limits of the tree protection barrier, including during barrier installation.
- 3. Install all tree protection prior to commencement of construction and remove when directed by the Engineer. Maintain protection at all times.
- 4. For closely spaced groups of trees, place the tree protection barrier around the entire group.
- 5. Inspect trunk protection and tree quarterly to prevent girdling. Adjust bands to allow tree growth as needed.
- 6. See plans for any additional requirements or modifications within the tree protection area.
- 7. Place weather resistant sign every 50' along the barrier, with 6" minimum text height and provide text in English and Spanish. Sign should read " Keep Out Tree Protection Area".
- 8. Alternate tree protection systems approved by the Engineer may be used in lieu of the tree protection barrier detailed on this Index as long as the critical root zone is protected.
- 9. The Critical Root Zone may be reduced, in the field, by a certified Arborist or Landscape Architect.







## PLAN ==== PROTECTION BARRIER FOR TREE GROUPINGS======



- 1. Trunk protection may be used when Tree Protection Barrier can not be reasonably erected, when approved by Engineer.
- 2. See Selective Clearing and Grubbing Plan for location of trunk protection, when applicable.
- 3. Adjust bands to allow tree growth (inspect quarterly to prevent girdling).

=  $\mathit{TRUNK}$   $\mathit{PROTECTION}$  =

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DESCRIPTION:



=TREE PROTECTION BARRIER=

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TREE PROTECTION AND PRESERVATION

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- 2. Mailboxes will not be permitted on Interstate highways, freeways, or other highways where prohibited by law or regulation.
- 3. The contractor shall give the Postmaster of the delivery route(s) written notice of project construction 7 days prior to the beginning of work, with Saturdays, Sundays and Holidays excluded.

The Contractor shall furnish and install one mailbox in accordance with this Index at each mail patron delivery location and maintain the box throughout the contract period. The Contractor shall apply box numbers to each patron box in accordance with identification specifications of the Domestics Mail Manual of the U. S. Postal Service; where local street names and house numbers are authorized by the Postmaster as a postal address, the Contractor shall inscribe the house number on the box; if the box is located on a different street from the patrons residence, the Contractor shall inscribe the street name and house number on the box.

The Contractor shall coordinate removal of the patrons existing mailboxes. Immediately after installing the new mailboxes the Contractor must notify each "Mail Delivery Patron" by Certified Mail that removal of the existing mailboxes must be accomplished in 21 days after receipt of notices. Patrons shall have the option of removing their existing mailboxes or leaving the mailboxes in place for removal by the Contractor; removal by the Contractor shall be included in the contract unit price for Mailbox, Each. The Contractor shall dispose of mailboxes and supports in areas provided by him.

Reuse of existing mailboxes by the Contractor will not be a requirement under any construction project; however where an existing mailbox meets the design requirements of this Index and is structurally and functionally sound, the Contractor at his option may elect to reuse the existing mailbox in lieu of constructing a new mailbox. Any use of existing mailboxes must be approved by the Engineer.

4. Mailboxes shall be light sheet metal or plastic construction, in traditional style only, and only in Size 1 as prescribed by the Domestic Mail Manual of the U. S. Postal Service (DMM).

Mailbox production standards, lists of approved manufacturers and suppliers of mailboxes, design approval and guidance may be obtained by writing to the Rural Delivery Division, Delivery Service Department, Operations Group, USPS Headquarters, Washington, DC 20260.

5. Mailboxes shall be located on the right-hand side of the roadway in the direction of the delivery route, except on one-way roads and streets where they may be placed on the left-hand side.

Mailboxes on rural highways shall be set with the roadside face of the box offset from the edge of the traveled way a minimum distance of the greater of the following:

- a. Shoulder width plus 8" to 12".
- b. 10' for ADT over 10,000 vpd.
   8' for ADT 100 to 10,000 vpd.
   6' for ADT under 100 vpd
   2'-6" for low speed and ADT under 100 vpd.

When a mailbox is installed within the limits of guardrail it should be placed behind the guardrail whenever practical.

Mailboxes on curbed highways, roads, and streets shall be set with the face of the box between 6" and 12" behind the face of curb. If the sidewalk abuts the curb or if an unusual condition exists which makes it difficult or impractical to install or serve boxes at the curb, the Contractor, with concurrence of the local postal authority, may be permitted to install all mailboxes at the back edge of the sidewalk, where they can be served by the carrier from the sidewalk.

- 6. Mailboxes shall be set with the bottom of the box between 42" and 48" above the mail stop surface, unless the U.S. Postal Service establishes other height restrictions.
- 7. No more than two mailboxes may be mounted on a support structure unless the support structure and mailbox arrangements have been shown to be safe by crash testing in accordance with NCHRP Report 350.

Neighborhood Delivery and Collection Box Units (NDCBU) are a specialized multiple mailbox installation that must be located outside the highway and street clear zones. The location of NDCBUs is the sole responsibility of the Postmaster for the delivery route under consideration.

8. Lightweight newspaper receptacles may be mounted below the mailbox on the side of the support post in conformance with the USPS Domestic Mail Manual. The mail patron shall be responsible for newspaper receptacle installation and maintenance.

9. Wood and steel support posts for both single and double mailbox mountings shall be embedded no more than 24" into the ground.

Concrete, block, brick, stone or other rigid foundation structure or encasement, either above or below the shoulder ground line, will not be permitted for mailboxes on rural highways. On urban roads and streets where mailbox support posts are set within rigid pavement back of curb, the support posts shall be separated from the pavement by a minimum of 1" of expansion material.

Support posts shall not be fitted nor installed with surface mount base plates.

10. At driveway entrances mailboxes shall be placed on the far side of the driveway in the direction of the delivery route.

At intersecting roads mailboxes shall be located 100' or more from the centerline of the intersecting road on the far side in the direction of the delivery route, with the distance increased to 200' when the route volume exceeds 400 vehicles per day.

11. Wood support posts shall be in conformance with the material and dimensional requirements of Section 952 and the treatment requirements of Section 955 of the Standard Specifications.

Steel support posts shall have an external finish equal to or better than two coats of weather resistant, air dried or baked, paint or enamel. Surface(s) shall be cleaned of all loose scale prior to finishing. The Postal Service prefers that posts be painted white, but other colors may be used when approved by the Engineer. When galvanized posts are used painting is not required.

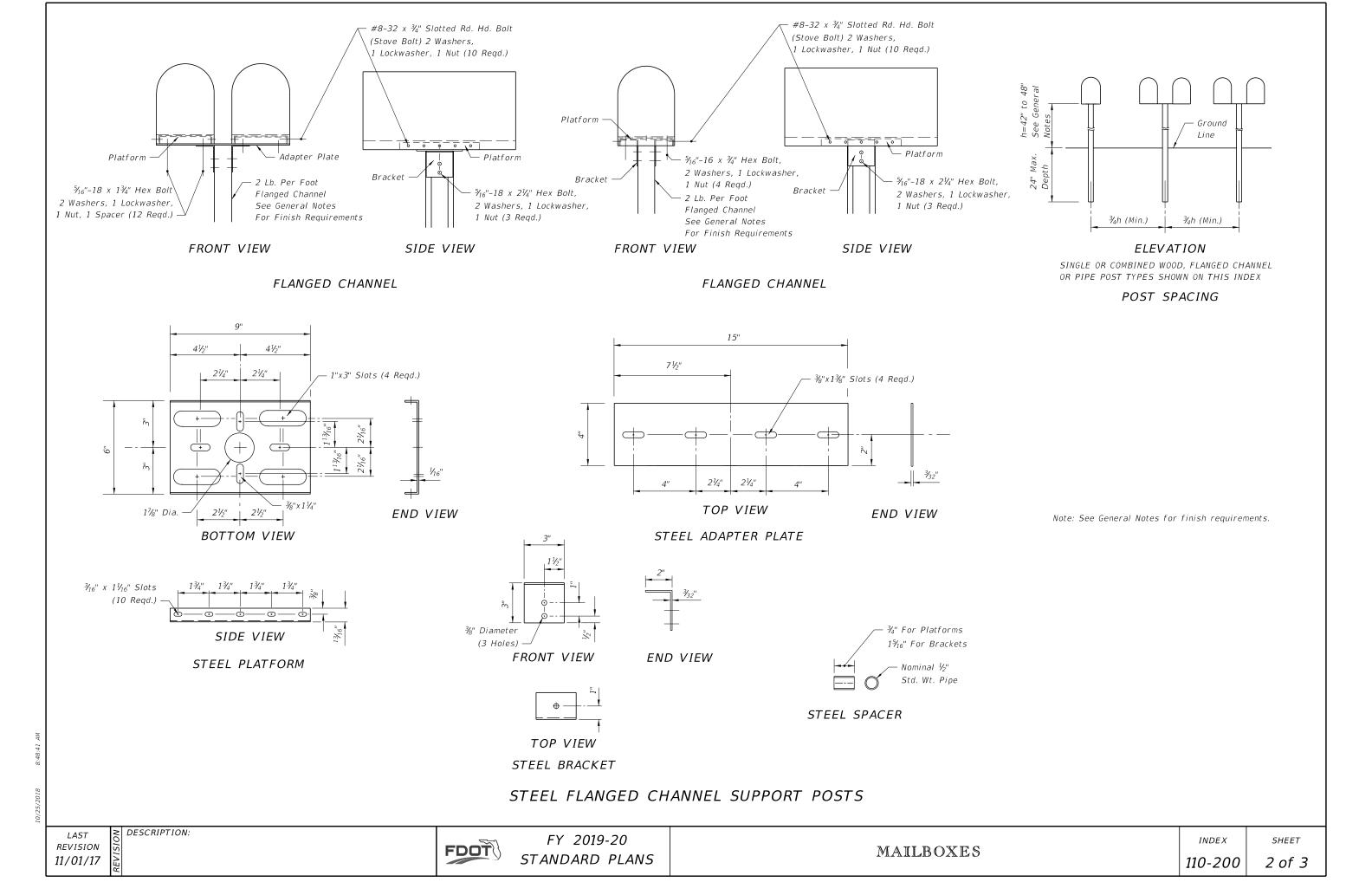
Mounting brackets, plates, platforms, shelves and accessory hardware surface finishes are to be suited to support post finish.

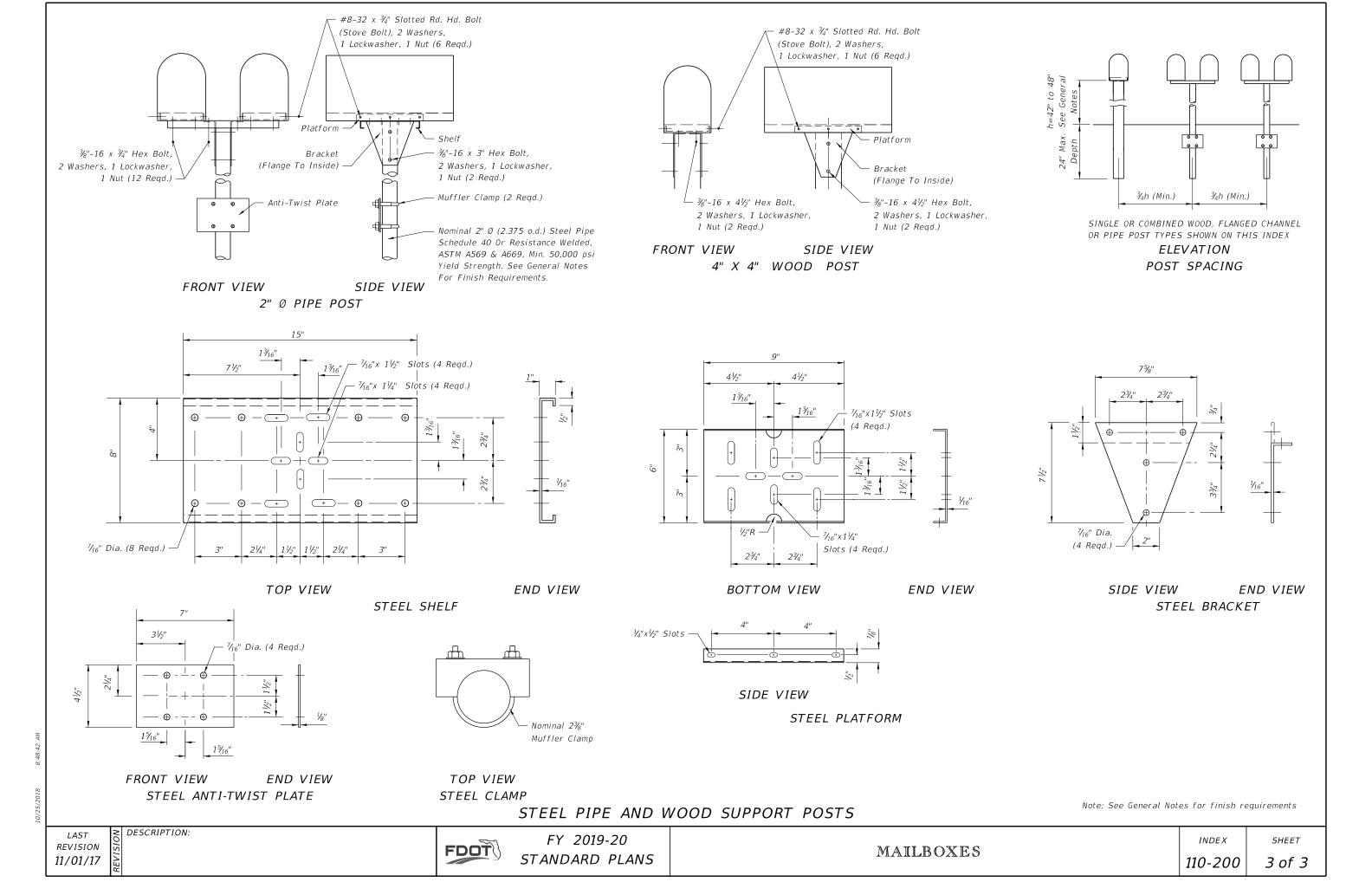
12. Mailboxes shall be paid for under the contract unit price for Mailboxes, Each. Payment shall be full compensation for boxes, posts and accessory items essential for installation in accordance with this standard; erection; adjustments to suit construction needs; and, for identification letters and numbers.

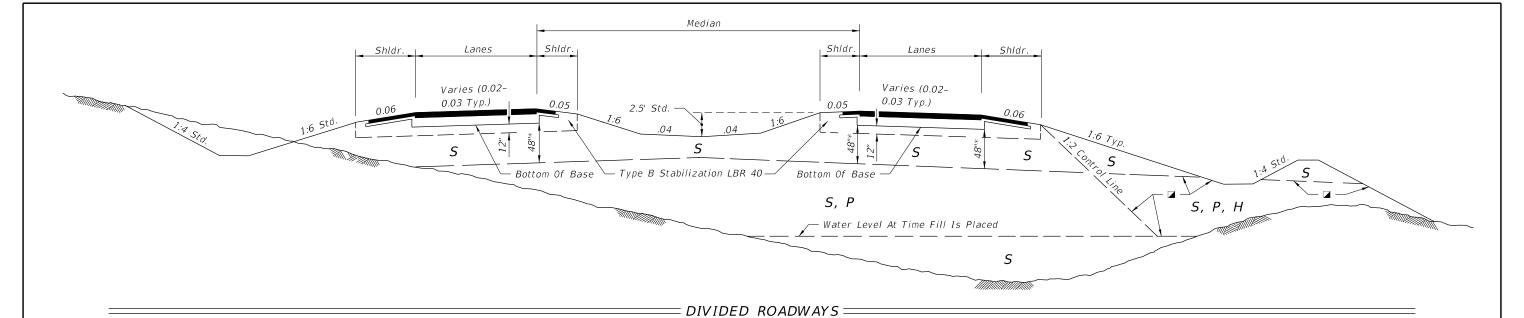
Payment shall be limited to one mailbox per patron address whether the mailbox is new, reused, salvaged, reset or relocated. Payment shall be per mailbox regardless of the number of mailboxes per support or grouping arrangement.

The above compensation shall include any work and cost incurred by the contractor for removal and disposal of existing mailboxes.

There shall be no payment participation for NDCBU furnishing, assembly, installation, resetting or relocation.



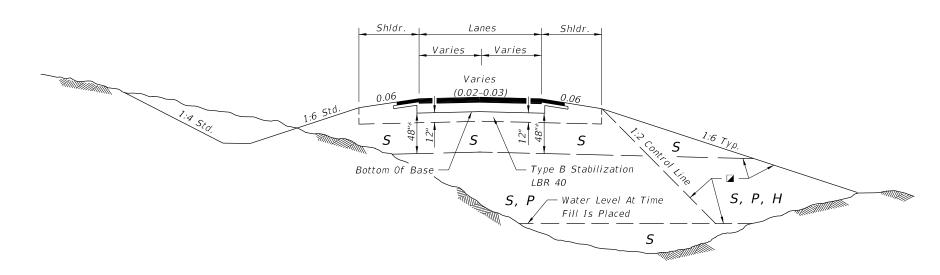




Excess Base

#### **GENERAL NOTES:**

- 1. Roadway dimensions are representative. Subgrade dimensions and control lines are standard. The details shown on this Index do not supersede the details shown in the Plans or Indexes 120-002 and 160-001.
- 2. Plastic (P) soils may be placed above the existing water level (at the time of construction) to within 4 feet of the proposed base. It should be placed uniformly in the lower portion of the embankment for some distance along the project rather than full depth for short distances.
- 3. High Plastic (H) soils excavated within the project limits may be used in embankment construction as indicated on this Index. High Plastic soils are not to be used for embankment construction when obtained from outside the project limits.
- 4. Select (S) soils having an average organic content of more than two and one-half (2.5) percent, or having an individual test value which exceeds four (4) percent, are not permitted in the subgrade portion of the roadbed. Select (S), Plastic (P), or High Plastic (H) soils having an average organic content of more than five (5) percent, or an organic content individual test result which exceeds seven (7) percent, are not permitted in the portion of embankment inside the control line, unless written authorization is provided by the District Geotechnical Engineer; these soils may be used for embankment construction outside the control line, unless restricted by the Plans or otherwise specified in the Plans, provided they can be compacted sufficiently to sustain a drivable surface for operational vehicles as approved by the Engineer. Determine average organic content from the test results from a minimum of three randomly selected samples from each stratum or stockpile of a particular material. Perform tests in accordance with AASHTO T 267 on the portion of a sample passing the No. 4 sieve.
- 5. Highly organic soils, composed primarily of partially decayed organic matter, often dark brown or black in color with an odor of decay, and sometimes fibrous, are designated as muck. Further, any stratum or stockpile of soil which contains pockets of highly organic material may be designated as Muck (M). Highly organic soils are not permitted within the subgrade or embankment portion of the roadbed.



#### = UNDIVIDED ROADWAY =

<u>SYMBOL</u>	<u>SOIL</u>	CLASSIFICATION (AASHTO M 145)
S	Select	A-1, A-3, A-2-4 **
Р	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL < 50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL > 50)
М	Muck	A-8

Classification listed left to right in order of preference.

- ☑ See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.
- ** Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction. They may be used in the subgrade portion of the roadbed when approved by the District Materials Engineer. A-2-4 material placed below the existing water level must be nonplastic and contain less than 15% passing the No. 200 U.S. Standard sieve.
- * For cut sections this dimension may be reduced to 24"; see Index 120-002. For minor collectors and local facilities this dimension may be reduced to 18".

## NOTES:

Friction Course ~ Surface Course

Base

- 1. All material in the shaded area is excess base to be removed.
- 2. There is no additional payment for removal of excess base material.

Neat Edge

Actual Limits of Base

#### = REMOVAL OF EXCESS BASE MATERIAL ==

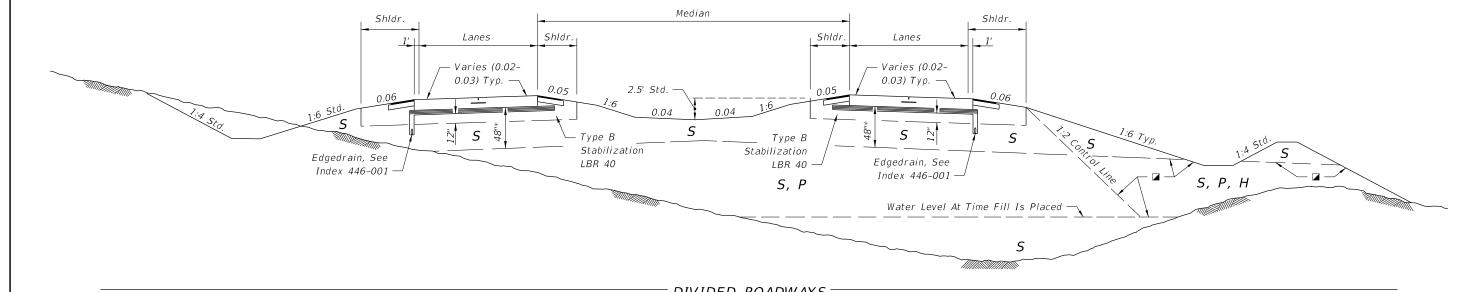
## DESCRIPTION:

REVISION 11/01/18

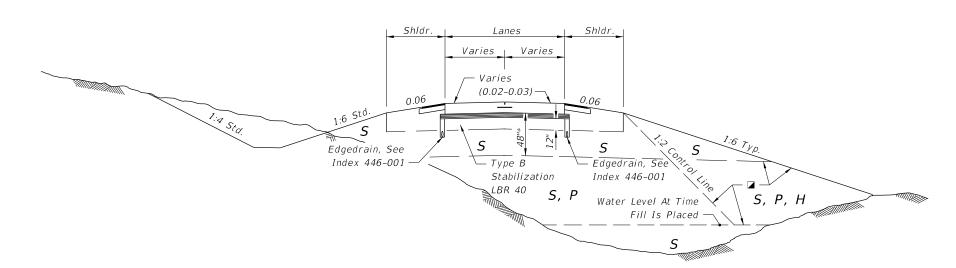
**FDOT** 

FY 2019-20 STANDARD PLANS

## GENERAL NOTES AND FLEXIBLE PAVEMENT



### = DIVIDED ROADWAYS=



#### = UNDIVIDED ROADWAY =

<u>SYMBOL</u>	<u>SOIL</u>	CLASSIFICATION (AASHTO M 145)
S	Select	A-1, A-3, A-2-4 **
Р	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL < 50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL > 50)
М	Muck	A-8

Classification listed left to right in order of preference.

☑ See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.

- ** Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction. They may be used in the subgrade portion of the roadbed when approved by the District Materials Engineer. A-2-4 material placed below the existing water level must be nonplastic and contain less than 15% passing the No. 200 U.S. Standard sieve.
- * For cut sections this dimension may be reduced to 24"; see Index 120-002. For minor collectors and local facilities this dimension may be reduced to 18".

RIGID PAVEMENT - ASPHALT BASE OPTION

**REVISION** 11/01/18

DESCRIPTION:

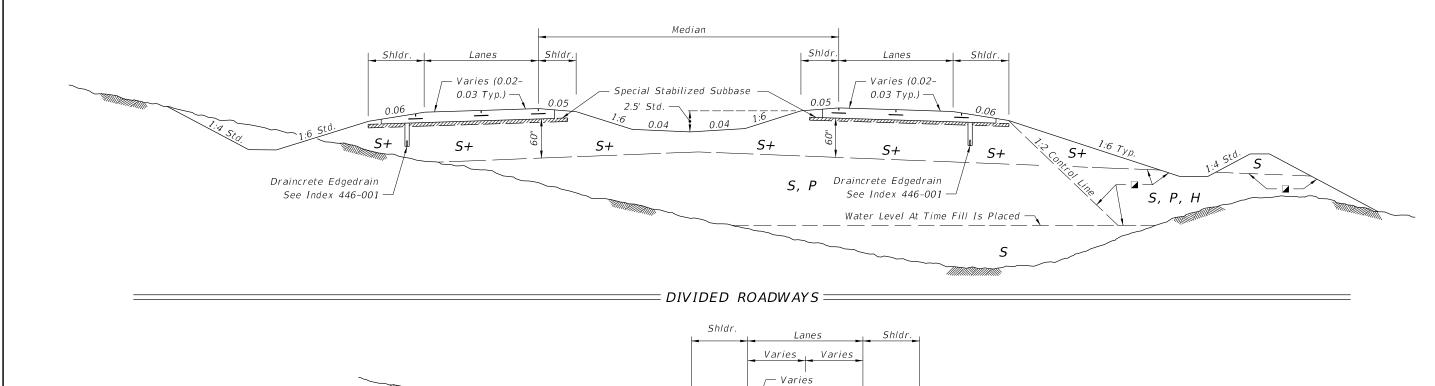
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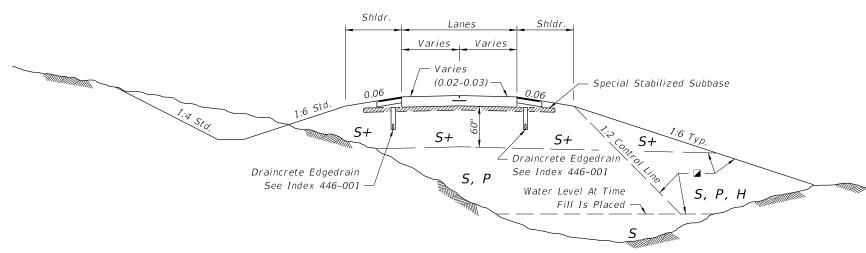
FY 2019-20 STANDARD PLANS

EMBANKMENT UTILIZATION

INDEX 120-001

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#### = UNDIVIDED ROADWAY =

<u>SYMBOL</u>	SOIL	CLASSIFICATION (AASHTO M 145)
S	Select	A-1, A-3, A-2-4 **
S+	Special Select	A-3 *** With Minimum Average Lab Permeability of $5 \times 10^{-5}$ cm/sec. (0.14 ft./day) as per AASHTO T 215
Р	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL<50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL>50)
М	Muck	A-8

Classification listed left to right in order of preference.

- ☑ See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.
- *** When called for in the Plans, some types of A-2-4 material may be approved in writing by the District Materials Engineer. This material must meet the minimum lab permeability requirement, be nonplastic, and not exceed 12% passing the No. 200 U.S. Standard sieve.
- ** Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction. A-2-4 material placed below the existing water level must be nonplastic and contain less than 15% passing the No. 200 U.S. Standard sieve.

Special Stabilized Subbase: 3" of #57 or #89 Coarse Aggregate Mixed Into Top 6".

RIGID PAVEMENT - SPECIAL SELECT SOIL OPTION

**REVISION** 11/01/18

DESCRIPTION:

FDOT

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EMBANKMENT UTILIZATION

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SHEET

Bottom Of Organic Material

Control Line Set By Normal Shoulder Point

Whether Or Not Shoulder Gutter Is Used

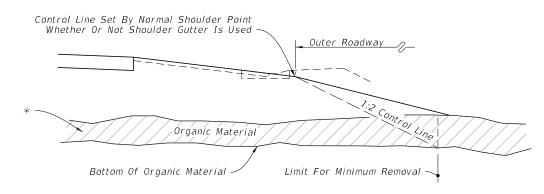
Overlying Material Organic Material

Outer Roadway

Limit For Minimum Removal -

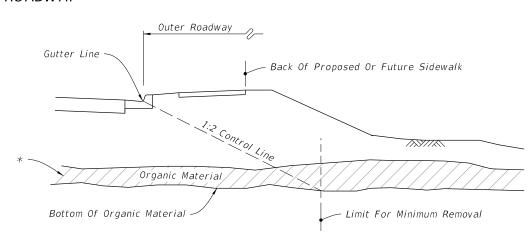
Back Of Proposed Or Future Sidewalk

Limit For Minimum Removal



## WITHOUT OVERBURDEN - HALF SECTION

### CONSTRUCTION OF FLUSH SHOULDER ROADWAY



WITHOUT OVERBURDEN - HALF SECTION

## CONSTRUCTION OF CURBED ROADWAY:

*Remove overlying material and organic material within the limits shown and backfill in accordance with Index 120-001, unless approved otherwise by the District Geotechnical Engineer; The limits include full median width when applied to divided facilities with median widths up to 64'; When median width is greater than 64' and for bifurcated roadways the organic material removal limits will be set by a 1:2 control line complimentary to the outer roadway that will accommodate one future median lane on each roadway unless specified otherwise by the plans.

#### GENERAL NOTES:

DESCRIPTION:

- 1. All details shown on this Index for removal of organic and plastic materials apply unless otherwise shown on the plans.
- 2. Utilize excavated materials in accordance with Index 120-001.
- 3. Where organic or plastic material is undercut, backfill with suitable material in accordance with Index 120-001, unless otherwise shown on the plans.
- 4. The term "Plastic Material" used in this Index in conjunction with removal of plastic soil is as defined under soil classifications for Plastic (P) and High Plastic (H) on Index 120-001.
- 5. See Index 160-001 for miscellaneous earthwork details.

- 6. The term "Organic Material" as used on this Index is defined as any soil which has an average organic content greater than five (5.0) percent, or an individual organic content test result which exceeds seven (7.0) percent. Remove organic material as shown on this Index and the plans unless directed otherwise by the District Geotechnical Engineer. Determine the average organic content from the test results from a minimum of three randomly selected samples from each stratum. Perform tests in accordance with AASHTO T267 on the portion of a sample passing the No. 4 sieve.
- 7. In areas of curbed roadway, where underdrain is to be constructed beneath the proposed pavement, the grade of the underdrain filter material will not extend above the bottom of the stabilized section of the subgrade. Gradation of the filter material must conform to Standard Specifications. The minimum grade of underdrain pipe is 0.2%.

GENERAL NOTES AND REMOVAL OF ORGANIC MATERIAL

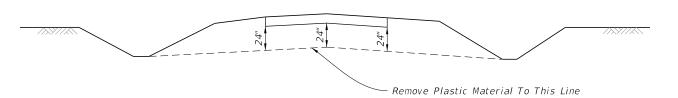
LAST **REVISION** 11/01/17

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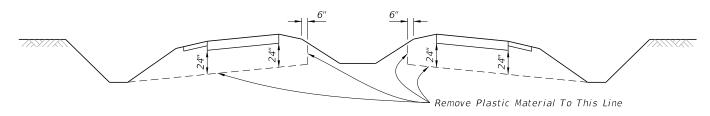
SUBSOIL EXCAVATION

120-002

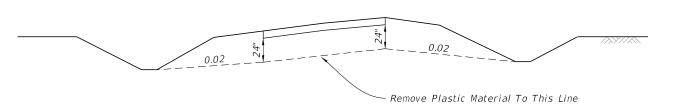
SHEET 1 of 2



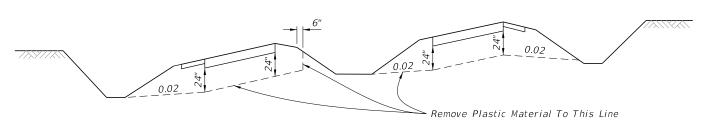
TYPICAL CUT SECTION ON TANGENT



TYPICAL CUT SECTION ON TANGENT



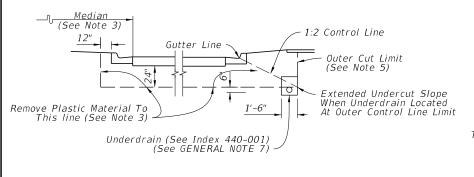
TYPICAL CUT SECTION ON SUPERELEVATION

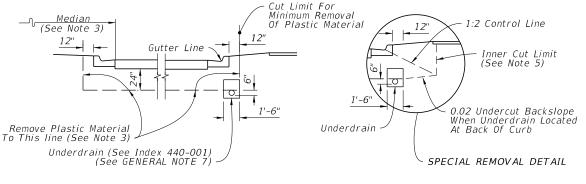


TYPICAL CUT SECTION ON SUPERELEVATION

=DIVIDED FREEWAYS, ARTERIALS, MAJOR COLLECTORS HAVING FLUSH === MEDIANS, ON UNDIVIDED ARTERIALS AND MAJOR COLLECTORS

=  $INTERSTATE\;FACILITIES$ , FREEWAYS,  $DIVIDED\;ARTERIALS$  =AND MAJOR COLLECTORS HAVING DEPRESSED MEDIANS





PREFERABLE REMOVAL

MINIMUM REMOVAL

NOTES:

- 1. See Sheet 1 for the GENERAL NOTES.
- 2. When the typical cut details are applied to minor collectors and local facilities, the undercut may be reduced from 24" to 18".
- 3. Where frequency of median breaks indicates that it is impractical to leave plastic material in the median, the designer may elect to indicate total removal of this material. If during construction it becomes apparent, due to normal required construction procedures, that it is impractical to leave the plastic material in the median, total removal of this material shall be approved by the Engineer.
- 4. Refer to roadway cross sections to determine whether minimum or preferable removal is used.
- 5. Where the Preferable Removal method is shown in the plans and it is impossible to place the underdrain at the Outer Cut Limit due to conflict with storm drain trunk lines, remove to Inner Cut Limit and place underdrain at location shown for Minimum Removal. (See Special Removal Detail)
- 6. Cross slopes of 0.02 shown above are minimums. Follow the cross slope of the pavement to the extent possible.

CONSTRUCTION AND LOCATION OF UNDERDRAIN IN CURBED ROADWAY (See Note 4)

REMOVAL OF PLASTIC MATERIAL

**REVISION** 11/01/17

DESCRIPTION:

FDOT

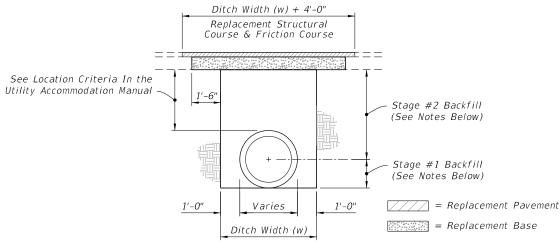
FY 2019-20 STANDARD PLANS

SUBSOIL EXCAVATION

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#### NOTES:

## PAVEMENT REMOVAL AND REPLACEMENT

- 1. Pavement shall be mechanically sawed.
- 2. The replacement asphalt shall match the existing structural and friction courses for type and thickness in accordance with current FDOT asphalt mix specifications.
- 3. The new base materials shall be either of the same type and composition as the materials removed or of equal or greater structural adequacy.

#### BACKFILL OPTION

#### 1. COMPACTED AND STABILIZED FILL

- A. Place backfill material in accordance with Specifications 125.
- B. In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.
- C. In Stage #2, construct compacted fill along the sides of the pipe and up to the bottom of the base, with the upper 12" receiving Type B Stabilization. In lieu of Type B Stabilization, the Contractor may construct using Optional Base Group 3.

#### 2. FLOWABLE FILL

- A. If compaction can not be achieved through normal mechanical methods then flowable fill may be used.
- B. Flowable fill is to be placed in accordance with Section 121 of the Specifications, as approved by the Engineer.
- C. Do not allow the utility being installed to float. If a method is provided to prevent flotation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.
- D. In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.
- E. In Stage #2, place flowable fill to the bottom of the existing base course.

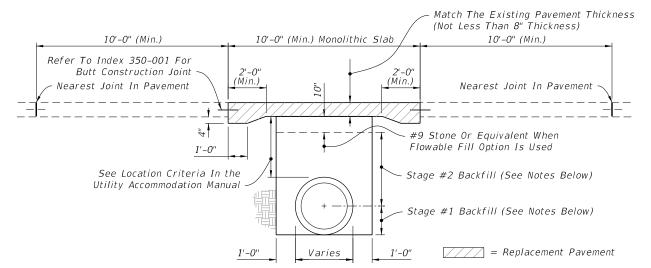
#### FLEXIBLE PAVEMENT CUT

#### GENERAL NOTES

- 1. The details provided in this Index apply to cases in which jack and bore or directional boring methods are not required by the Engineer.
- 2. Flowable fill shall not be placed directly over loose, or high plastic, or muck material (see Index 120-001) which will cause settlement due to fill weight. Where highly compressible material exists, the amount, shape and depth of flowable fill must be engineered to prevent pavement settlement.
- 3. These details do not apply to utility cuts longitudinal to the centerline of the roadway which may require the additional use of geotextiles, special bedding and backfill, or other special requirements.
- 4. Method of construction must be approved by the Engineer

DESCRIPTION:

5. Some pipe may require special granular backfill up to 6" above top of pipe. Geotextiles may be required to encapsulate the special granular material.



#### NOTES:

#### PAVEMENT REMOVAL AND REPLACEMENT

- 1. High early strength cement concrete (3000 psi) meeting the requirements of Standard Specification 346 shall be used for rigid pavement replacement.
- 2. Pavement shall be mechanically sawed and restored to conform with existing pavement joints within 12 hours. (See Index 350-001)

#### BACKFILL OPTION

#### 1. GRANULAR BACKFILL

- A. Any edgedrain system that is removed shall be replaced with the same type materials. Any edgedrain system that is damaged shall be repaired with methods approved by the Engineer.
- B. Fill material shall be placed in accordance with the Standard Specifications. Fill material shall be special select soil in accordance with Index 350-001.
- C. In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.
- D. In Stage #2, construct fill along the sides of the pipe and up to the bottom of replacement pavement.

#### 2. FLOWABLE FILL

- A. If mechanical compaction can not be achieved through normal mechanical methods then flowable fill may be used.
- B. Flowable fill is to be placed in accordance with Section 121 of the Specifications, as approved by the Engineer.
- C. Do not allow the utility being installed to float. If a method is provided to prevent flotation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.
- D. In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.
- E. In Stage #2, place flowable fill to the bottom of the stone layer.

#### =RIGID PAVEMENT CUT=

- 6. Where asphalt concrete overlays exist over full slab concrete pavement, the replacement pavement shall have an overlay constructed over the replacement slab. The overlay shall match the existing asphalt pavement thickness. The replacement friction course shall match the existing friction course, except structural course may be used in lieu of dense graded friction course.
- 7. All shoulder pavement, curb, curb and gutter, and their substructure disturbed by utility trench cut construction shall be restored in kind.
- 8. The use of flowable fill to reduce the time traffic is taken off a facility is acceptable but must have prior approval by the Engineer. Flowable fill use is allowed only when properly engineered for pavement crossings, whether straight or diagonal, and shall not be installed for significant depths or lengths. The maximum length shall be fifty (50) feet and a maximum depth of six (6) feet unless supported by an engineering document prepared by a registered professional engineer that specializes in soils engineering. The engineering document shall address the evaluation of local groundwater flow interruption and settlement potential.
- 9. Excavatable flowable fill is to be used when the flowable fill option is selected.

#### TRENCH CUTS AND RESTORATIONS ACROSS ROADWAYS

LAST REVISION 11/01/17

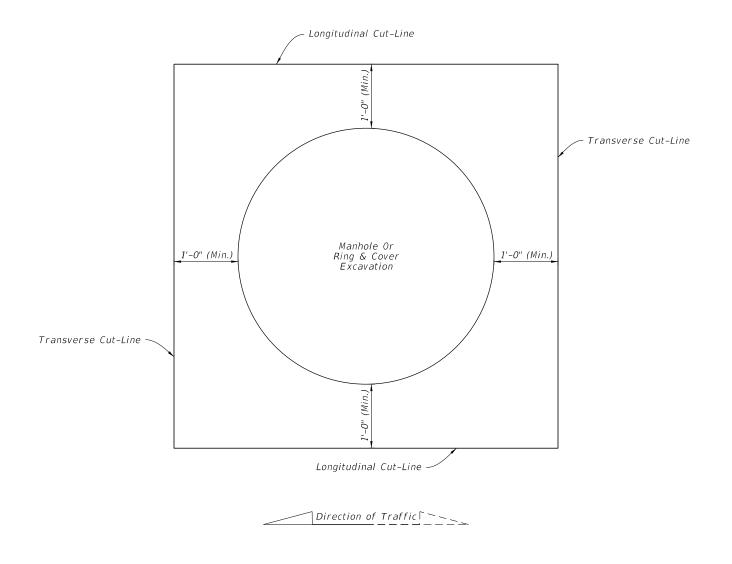
FDOT

FY 2019-20 STANDARD PLANS

UTILITY ADJUSTMENTS THRU EXISTING PAVEMENT

INDEX

SHEET



#### = PARTIAL CUTS FOR RING AND COVER ADJUSTMENTS ==

#### NOTES

- 1. Cut-Lines must be straight and cleanly sawed.
- 2. See Sheet 1 for replacement pavement.
- 3. Adjust manholes prior to placing friction course when pavement resurfacing is occurring in the area adjacent to the manhole.
- 4. Align Longitudinal Cut-Lines with pavement joint or center of traffic lane to avoid wheel path.
- 5. For rigid pavement, align Transverse Cut-Lines with nearest existing joint.

## NONTRENCH PAVEMENT CUTS FOR UNDERGROUND UTILITY STRUCTURES IN PAVEMENT

REVISION 11/01/17

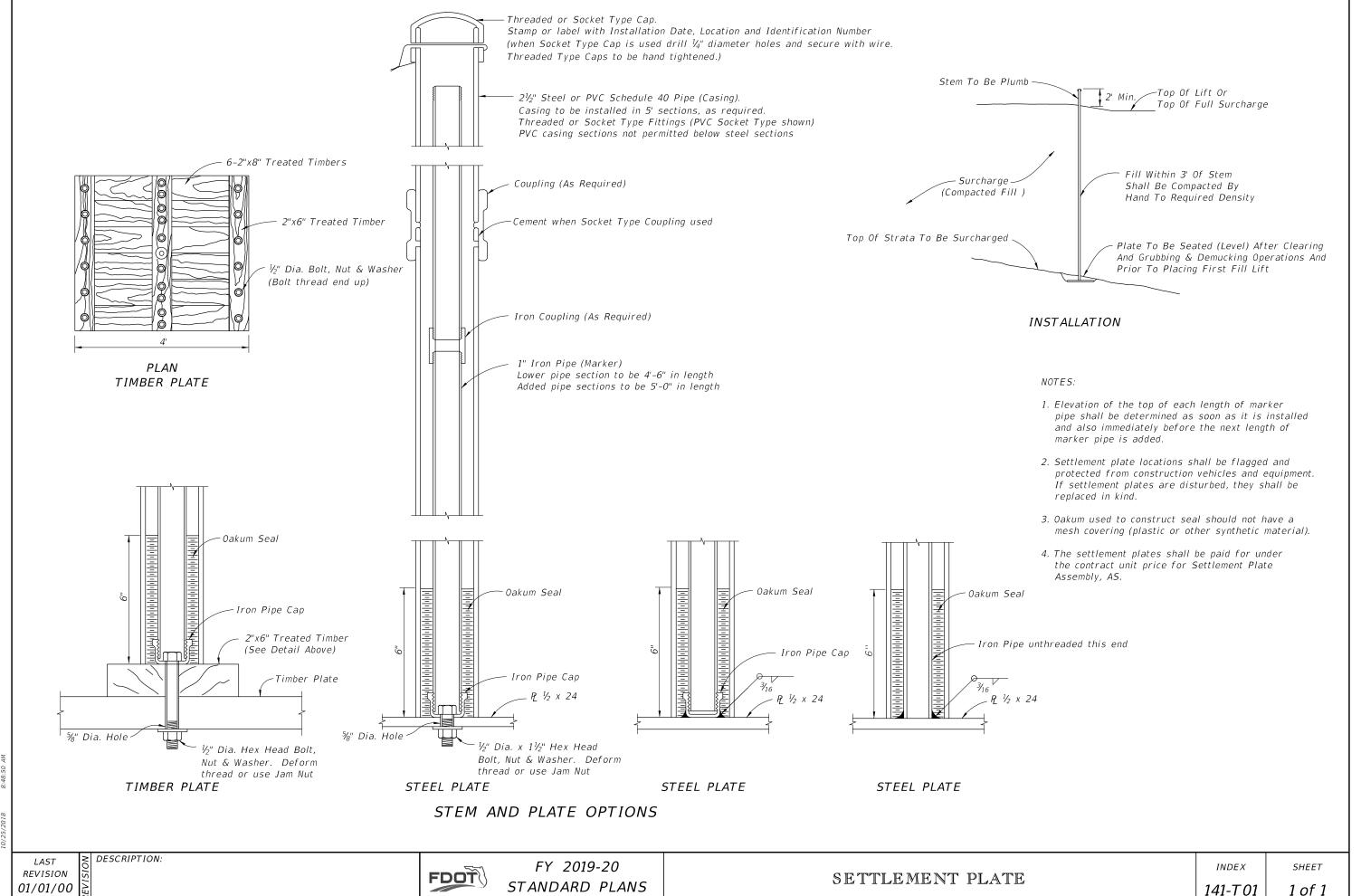
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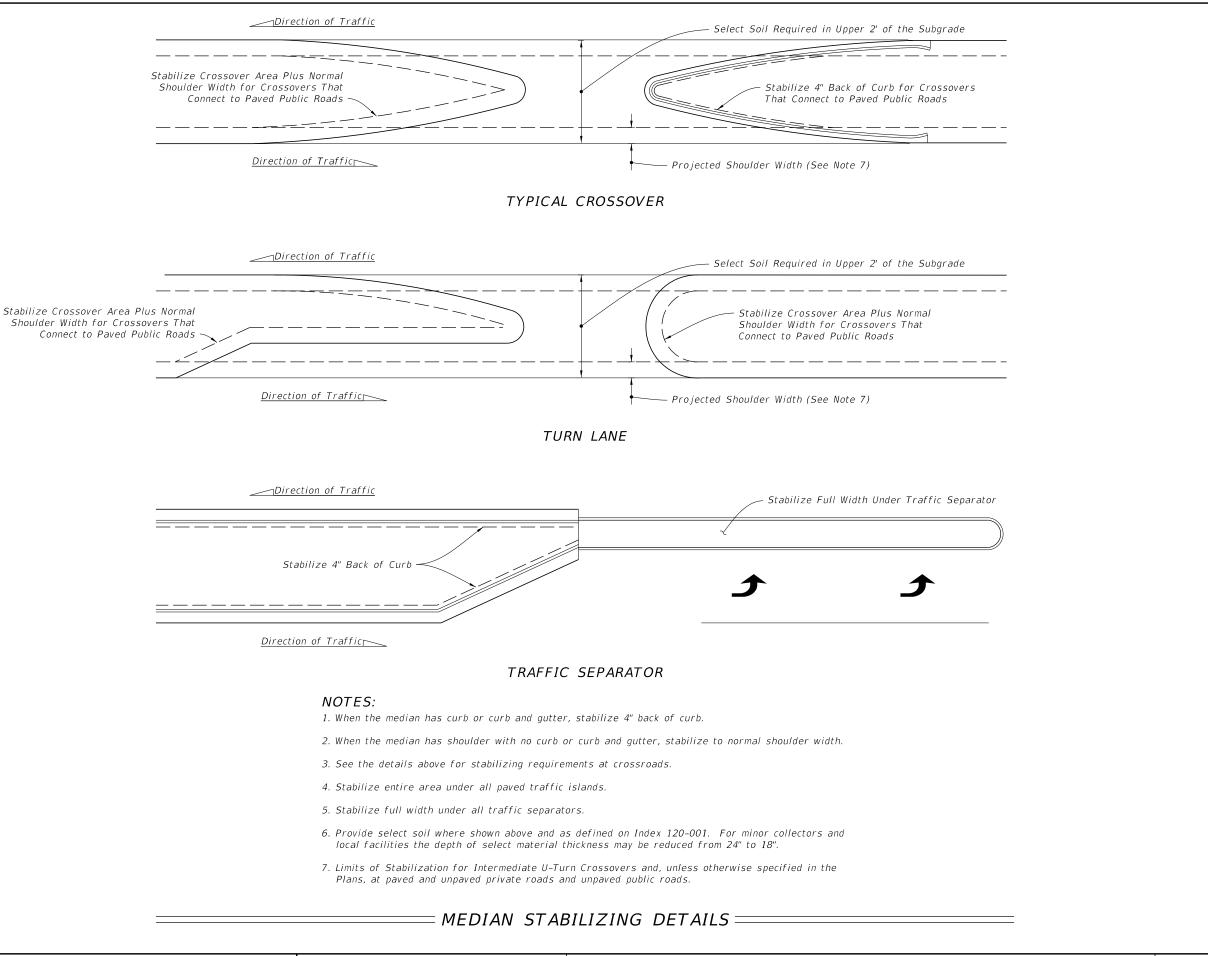
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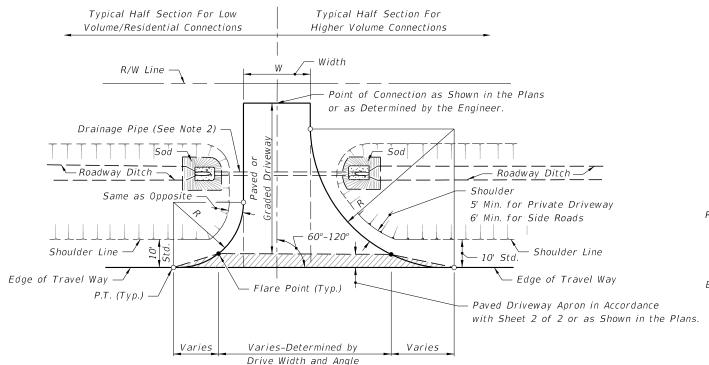
UTILITY ADJUSTMENTS THRU EXISTING PAVEMENT

INDEX 125-001 SHEET

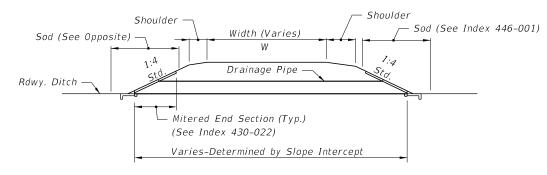
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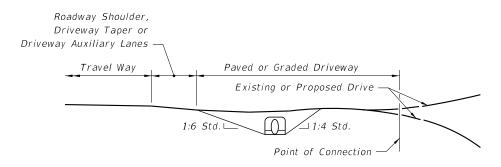




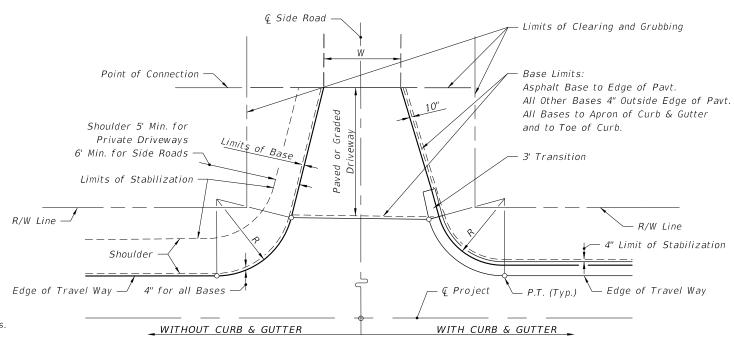
#### PLAN



#### DRAINAGE SECTION



#### DRIVEWAY PROFILE AND END VIEW



= LIMITS OF CLEARING & GRUBBING.=STABILIZING AND BASE AT DRIVEWAYS

PLAN

#### DRIVEWAY ENTRANCES NOTES:

- 1. See Plans for Driveway Width (W) and Return Radius (R).
- 2. See the Plans for drainage pipe size and length or as determined by the Engineer. The size will be no less than 15" diameter or equivalent.
- 3. Stable material may be required for graded driveways to private property as directed by the Engineer in accordance with Specification 102-8.
- 4. The driveway pavement requirement at graded connections may be waived for connections serving one or two homes or field entrances with less than 20 trips per day, or 5 trips per hour as approved by the Engineer, or when not shown in the Plans.

#### 5. Point of Connection:

- a. Construct paved driveways for all paved connecting facilities. The connecting point will be determined by the Engineer.
- b. Construct paved driveways for all business, commercial, industrial or high volume residential graded connecting facilities. Construct the connecting point 30'-0' from edge of travel way or at R/W line, whichever is less.
- c. Construct paved driveways for all side road connections. The R/W is the connecting point.

**REVISION** 11/01/18

DESCRIPTION:



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# DRIVEWAY TYPES =

# AREAS FOR ONE 5' DEEP

	DRIVEV	VAY APRO	ON (SY)			
Drive		Inters	section			
Width	Noi	rmal	Ske	Skewed		
(Ft.)	Type I	Type II	Type I	Type II		
12	26	51	31	60		
14	27	52	33	61		
16	28	53	34	63		
18	29	54	35	64		
20	31	55	37	65		
22	32	56	38	67		
24	33	57	39	68		
26	34	58	40	69		
28	35	59	42	70		
30	36	61	43	72		
32	37	62	44	73		
34	38	63	46	74		
36	39	64	47	76		
38	41	65	48	77		
40	42	66	49	78		
42	43	67	51	79		
44	44	68	52	81		
46	45	69	53	82		
48	46	71	55	83		
50	47	72	56	85		
52	48	73	57	86		
54	49	74	58	87		
56	51	75	60	88		
58	52	76	61	90		
60	53	77	62	91		

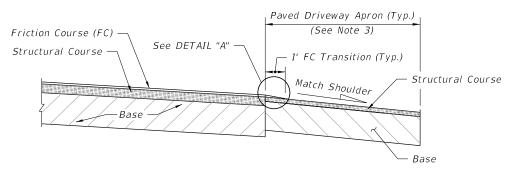
## MATERIAL TYPES AND THICKNESSES FOR PAVED CONNECTIONS

6	Ad-t-ni-l-	Minimum Thickness (in.)		
Course	Materials	Connections	Roadway*	
Structural	Asphaltic Concrete	1 1/2"	11/2"	
Bases	Optional Base (See Specification 285)	0.B.G. 2	0.B.G. 3	

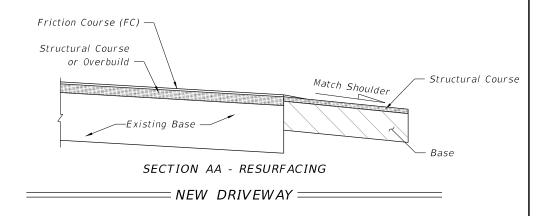
* Travel way flares (bypass lanes), auxiliary lanes serving more than a single connection, and all median crossovers including their auxiliary lanes and/or transition tapers.

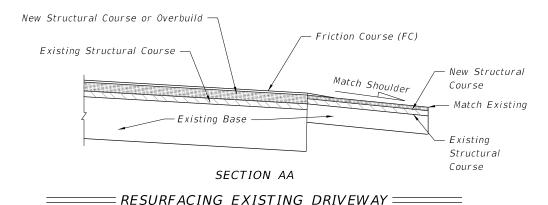
## **NOTES**

- 1. Use same material for driveway structural course and roadway overbuild or structural course, except as approved by the Engineer for graded connections. Other Department-approved equivalent pavements may be used at the discretion of the Engineer.
- 2. Auxiliary lanes and their transition tapers shall be the same structure as the abutting travel way pavement thickness or any of the roadway structures tabulated above, whichever is thicker.
- 3. If an asphalt base course is used for a driveway, its thickness may be increased to match the edge of travel way pavement thickness in lieu of a separate structural course. 6" of Portland cement concrete will be acceptable in lieu of the asphalt base and structural courses. See Notes 4 and 5 below.
- 4. A structural course is required for flexible pavements when they are used for auxiliary lanes serving more than a single connection.
- 5. Use Class NS concrete at least 6" thick for driveways paved with Portland Cement Concrete. Construct in accordance with Specifications 347, 350, and 522.
- 6. The Department may require other pavement criteria where local conditions warrant.



#### SECTION AA - NEW CONSTRUCTION





#### GENERAL NOTES:

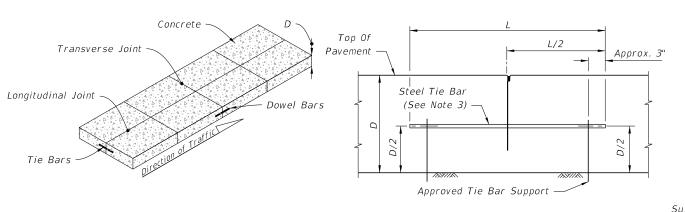
- 1. Driveways are to be constructed or resurfaced for low volume (single family, duplex, farm, etc.) residential connections as directed by the Engineer.
- 2. Driveways construction is not required for low volume residential connections where roadway shoulders are paved.
- 3. Match existing paved shoulder widths  $\geq 4'$ . For all other shoulders conditions, construct at 5' wide.
- 4. Connections beyond the shoulder width are to be constructed as directed by the Engineer.
- 5. Construct Driveway Base in accordance with Specification 286.
- 6. Payment for structural course and friction course is to be included in roadway pavement pay item.

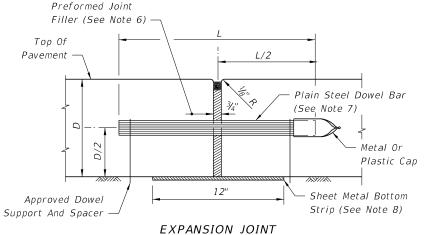
**REVISION** 11/01/18

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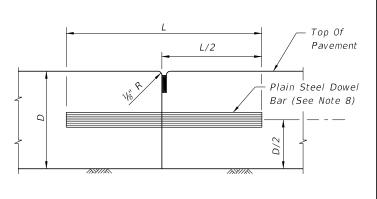
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(See Note 6)

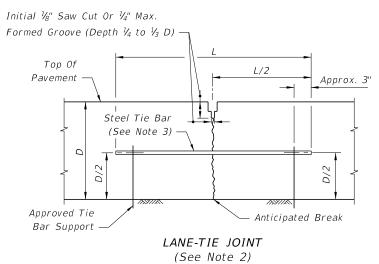


#### NOTES:

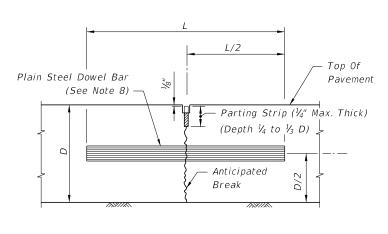
- 1. For joint seal dimensions see Sheet 2.
- 2. For slabs poured simultaneously, tie bars may be inserted in the plastic concrete by means approved by the Engineer.
- 3. For Longitudinal Joints:
- A. Tie bars are deformed #4 or #5 reinforcing steel bars meeting the requirements of Specification 931.
- B. Provide a standard load transfer tied joint with #4 bars 25" in length at 24" spacing or #5 bars 30" in length at 38" spacing.
- 4. Transverse joints are to be spaced at a maximum of 15'. Dowels are required at all transverse joints unless otherwise noted in the plans.
- 5. Expansion joints to be placed at street intersections and other locations as indicated in the Plans. For bridge expansion joints, see Index 370-001.
- 6. Punch clean holes in preformed joint filler greater than bar diameter.
- 7. Coat and lubricate plain steel dowel bars in accordance with Specification 350.
- 8. Sheet metal bottom strips in accordance with Specification 931.

DESCRIPTION:

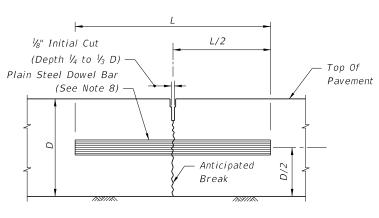
BUTT CONSTRUCTION JOINT



LONGITUDINAL JOINTS



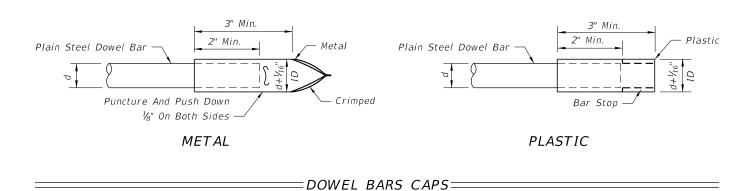
BUTT CONSTRUCTION JOINT (Used At Discontinuance Of Work)

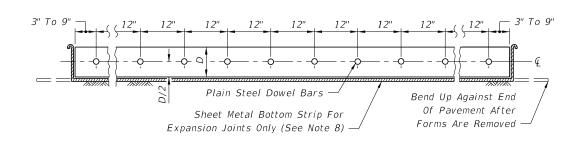


CONTRACTION JOINT (Vibro Case Method) CONTRACTION JOINT (Sawed Method)

TRANSVERSE JOINTS

DOWELS (LEN	IGTH 18")
Pavement Thickness "D"	Diameter
6"-6 ¹ / ₂ "	3/4"
7"-8"	1"
8½"-10½"	1 ½"
≥11"	1½"





DOWEL BAR LAYOUT=

**REVISION** 11/01/18

**FDOT** 

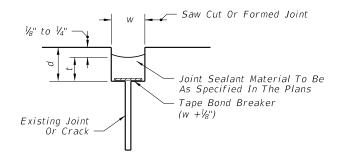
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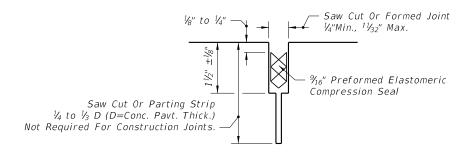
INDEX 350-001

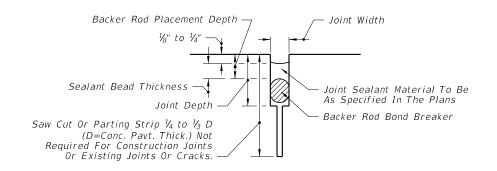
SHEET

CONCRETE PAVEMENT JOINTS

1 of 4







Note: Dimension w will be shown in the plans or established by the Engineer based on field conditions. Dimension d will be constructed so that the shape factor w/t has a maximum value of 2.0 and a minimum value of 1.0.

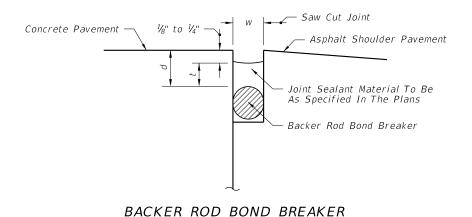
FOR NEW PROJECTS PREFORMED ELASTOMERIC COMPRESSION SEAL

FOR NEW AND REHABILITATION PROJECTS BACKER ROD BOND BREAKER

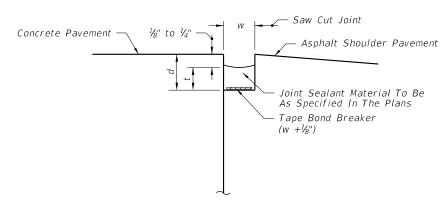
FOR REHABILITATION PROJECTS TAPE BOND BREAKER

CONCRETE-CONCRETE JOINTS

 $d = w = \frac{3}{4}$ " Unless Specified Otherwise In The Plans



 $d = w = \frac{3}{4}$ " Unless Specified Otherwise In The Plans



TAPE BOND BREAKER

FOR NEW AND REHABILITATION PROJECTS; EITHER TAPE OR BACKER ROD BOND BREAKER REQUIRED; SHOULDER MUST BE REPAIRED IF PROPER JOINT SHAPE CAN NOT BE ATTAINED

CONCRETE-ASPHALT SHOULDER JOINTS

BACKER	ROD	BOND	BR	REAKER
(CONCRET	F-CC	NCRFT	F	IOINTS)

	JOINT	DIMENSION	S (INCHES)	
JOINT	SEALANT BEAD	BACKER	MINIMUM JOINT	BACKER ROD PLACEMENT
WIDTH	THICKNESS	ROD DIA.	DEPTH	DEPTH
1/4	1/4	3/8	1	1/2
3/8	1/4	1/2	1 1/4	1/2
1/2	1/4	5/8	1 1/4	1/2
5/8	⁵ / ₁₆	3/4	11/2	% ₁₆
3/4	3/8	1	13/4	5/8
7/8	7∕ ₁₆	11/8	13/4	11/16
1	1/2	1 1/4	2	3/4
>1	1/2	11/4+	2+	3/4

Unless otherwise indicated on the plans the joint width for new construction will be 1/4" for construction joints,  $\frac{3}{8}$ " for all other joints.

For rehabilitation projects the joint width will be shown on the plans or established by the Engineer based on field conditions.

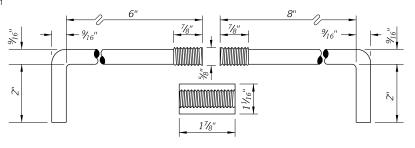
JOINT SEAL DIMENSIONS

**REVISION** 11/01/17

DESCRIPTION:

**FDOT** 

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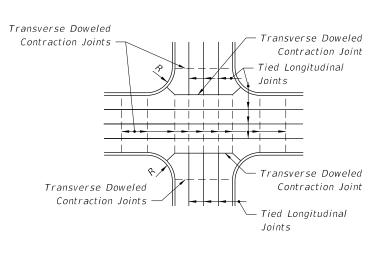
Note: After the concrete has set to the extent that the Keyway will retain its shape, the hex bolt and plastic insert shall be removed. The remaining portion of the hook bolt assembly shall be installed immediately prior to placing of concrete in the adjacent lane.

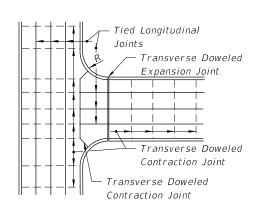
Anchor bolts shall be Grade C in accordance with ASTM A 307.

Threaded sleeves shall develop the full strength of the bolt and meet the material and thread requirements of ASTM A 563.

#### ALTERNATE KEYWAY AND HOOK BOLT

# STEEL HOOK BOLT ASSEMBLY





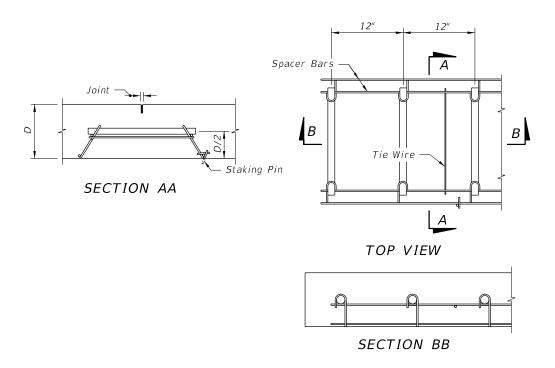
JOINT LAYOUT AT THRU INTERSECTION

JOINT LAYOUT AT 'T' INTERSECTIONS

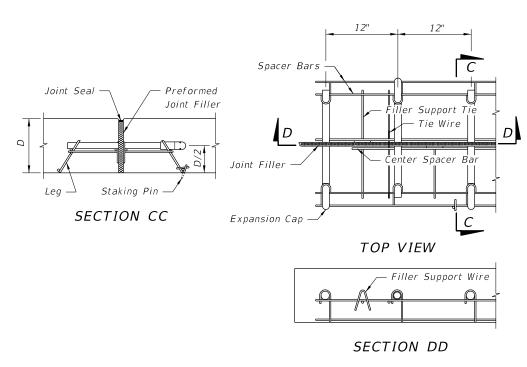
# JOINT ARRANGEMENT

## NOTES

- 1. Longitudinal joints will not be required for single lane pavement 14' or less in width. For entrance and exit ramp joint details, see Sheet 4.
- 2. Arrangement of longitudinal joints are to be as directed by the Engineer.
- 3. All manholes, meter boxes and other projections into the pavement shall be boxed-in with ½" preformed expansion joint material.



# CONTRACTION ASSEMBLY



# EXPANSION ASSEMBLY

Note: Proprietary contraction and expansion assemblies may be used. Products shall be introduced to the State Construction Office in accordance with section (C) of the Product Evaluation Procedure.

**REVISION** 11/01/18

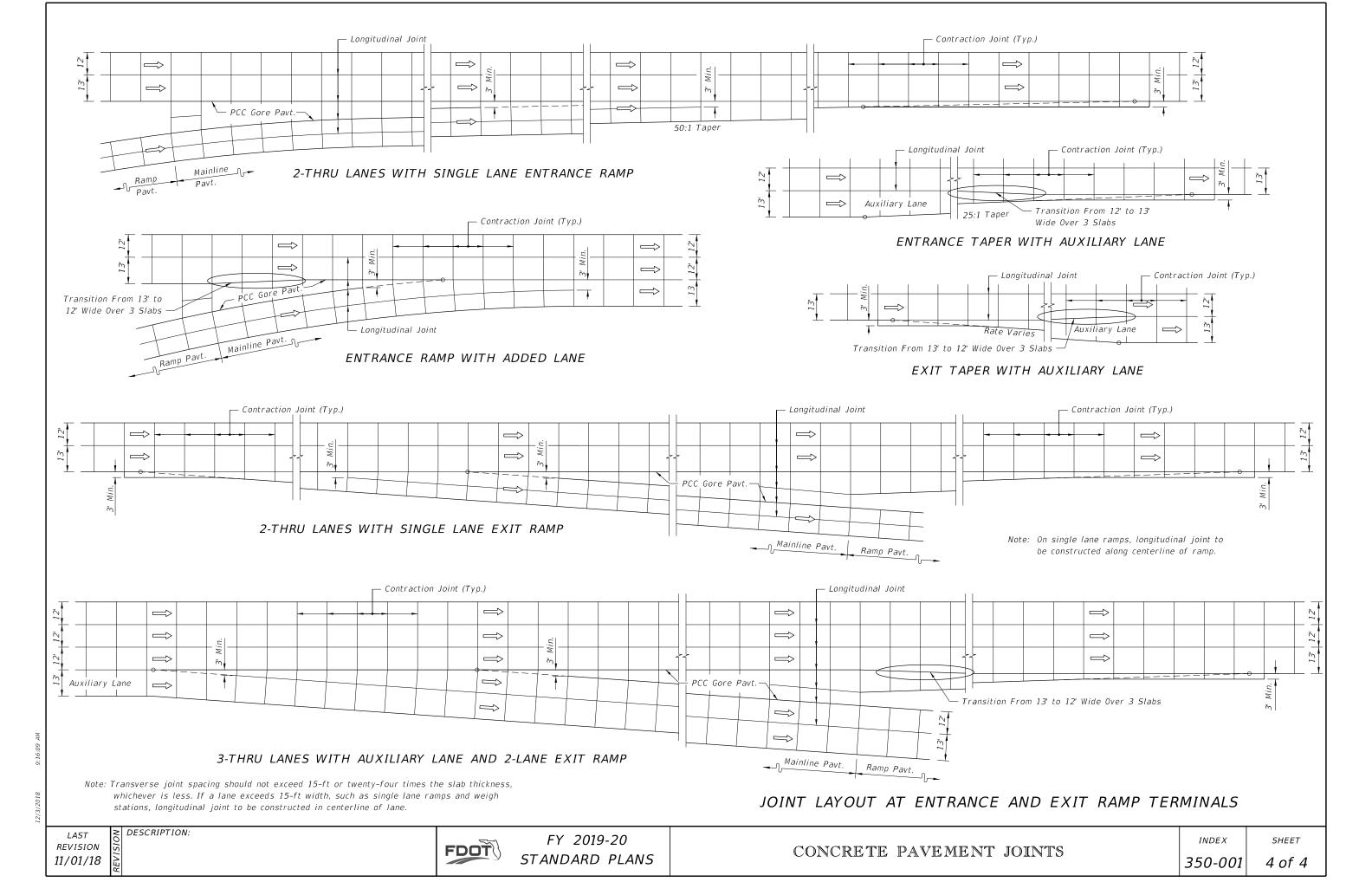
DESCRIPTION:



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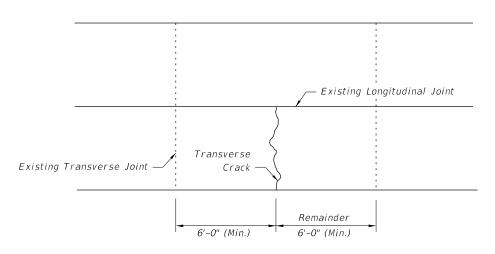


FIGURE 10.2 - REPAIR METHOD: NONE OR CLEAN AND SEAL

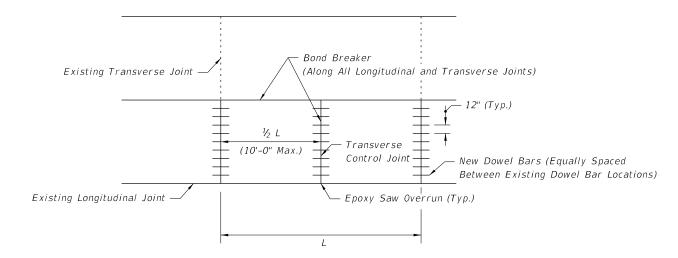


FIGURE 10.3 - FULL SLAB FULL DEPTH REPLACEMENT

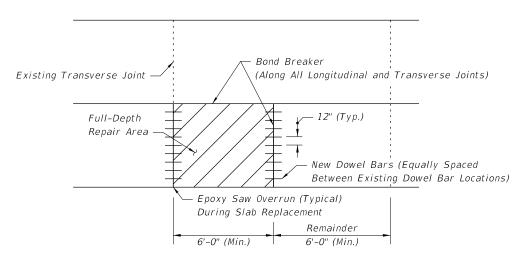


FIGURE 10.4 - PARTIAL SLAB FULL DEPTH REPLACEMENT

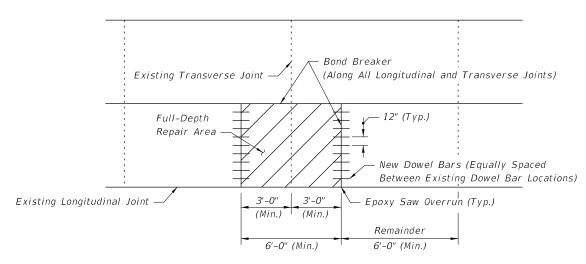


FIGURE 10.5 - FULL-DEPTH REPAIR ON BOTH SIDES OF THE JOINT

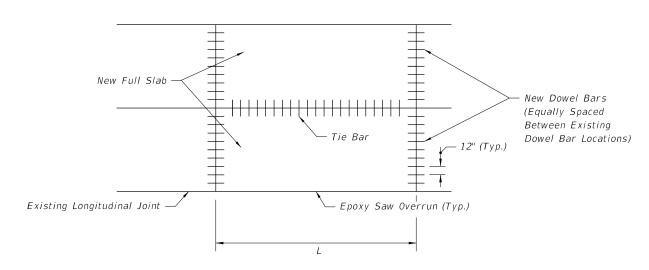


FIGURE 10.6 - MULTIPLE SLAB FULL DEPTH REPLACEMENT

#### GENERAL NOTES

- 1. For Repair and Replacement Criteria see Sheet 2.
- 2. Full depth repairs consist of removing and replacing at least a portion of the existing slab to the bottom of the concrete.
- 3. Repair boundaries shall be sawed full-depth with diamond saw blades. On hot days, it may not be possible to make this cut without first making a wide, pressure relief cut within the repair boundaries. A carbide-tipped wheel saw may be used for this purpose, but the wheel saw must not intrude on the adjacent lane, unless the lane is slated for repair. The wheel saw cuts produce a ragged edge that promotes excessive spalling along joints. Hence, if wheel saw cuts are made, diamond saw cuts must be made 18 in. outside the wheel saw cuts. To prevent damage to the base, the wheel saw must not be allowed to penetrate more than 0.5 in. into the base.
- 4. No additional base or subgrade material shall be added and all loose base or subgrade material shall be removed prior to placement of the new concrete slab. The concrete slab shall be placed to the full depth of the material removed. No additional compensation will be allowed for additional concrete required to bring proposed concrete slab up to finished grade.
- 5. Removal of the damaged concrete pavement shall be by lifting. Any good concrete pavement which is damaged during removal of damaged areas shall be removed and replaced by the contractor at his expense.
- 6. If the roadway contract includes grinding, then the slab replacement shall be performed first.
- 7. During slab replacement operations, fill any saw cut over runs into adjacent slabs with epoxy.
- 8. Install tie bars at longitudinal joints when two full adjacent or multiple replaced slabs.

**REVISION** 11/01/17

DESCRIPTION:

**FDOT** 

FY 2019-20 STANDARD PLANS CONCRETE SLAB REPLACEMENT

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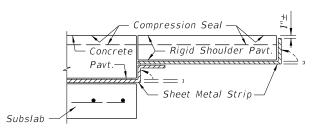
# SLAB REPAIR AND REPLACEMENT CRITERIA

DISTRESS PATTERN		SEVERITY/DESCRIPTION	REPAIR METHOD	REFERENCE
CRACKING				
	Light	$<\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!$	None	Figure 10.2
Longitudinal	Moderate	$\frac{1}{8}$ " <width <3"="" <\frac{1}{2}",="" spalling="" td="" wide<=""><td>Clean and Seal</td><td>Figure 10.2</td></width>	Clean and Seal	Figure 10.2
	Severe	width $>\frac{1}{2}$ ", spalling $>3$ " faulting $>\frac{1}{2}$ "	Replace	Figure 10.3
	Light	$<\!V_{\!8}$ ", no faulting, spalling $<\!V_{\!2}$ " wide	None	Figure 10.2
Transverse	Moderate	$\frac{1}{8}$ " <width <<math="">\frac{1}{2}", spalling &lt;3" wide</width>	Clean and Seal	
	Severe	width $>\frac{1}{2}$ ", spalling $>3$ " faulting $>\frac{1}{2}$ "	Replace	Figure 10.3, 10.4 and 10.5
Corner Breaks	adjacent lo	the slab is separated by a crack that intersects the ngitudinal and transverse joint, describing an approximate ith the direction of traffic.	Full Depth	Figure 10.4 and 10.5
Intersecting Random Cracks (Shattered Slab)	Cracking pa	tterns that divide the slab into three or more segments.	Full Depth	Figure 10.3 and 10.4
JOINT DEFICIENCIES				
	Light	spall width $<1\frac{1}{2}$ ", $<\frac{1}{3}$ slab depth, $<12$ " in length	None	Figure 10.4 and 10.5
Spall Nonwheel Path	Moderate	$1\frac{1}{2}$ " <spall <="" <3",="" <math="" width="">\frac{1}{3} slab depth, &lt;12" in length</spall>	None	Figure 10.4 and 10.5
	Severe	spall width >3" or length >12"	Full Depth	Figure 10.4 and 10.5
	Light	spall width $<1\frac{1}{2}$ ", $<$ than $\frac{1}{3}$ slab depth, $<$ 12" in length	None	Figure 10.4 and 10.5
Spall Wheel Path	Moderate	$1\frac{1}{2}$ " < spall width < 3", < $\frac{1}{3}$ slab depth, < 12" in length	Full Depth	Figure 10.4 and 10.5
	Severe	spall width >3" or length >12"	Full Depth	Figure 10.4 and 10.5
SURFACE DETERIORATION				
Pop Outs Nonwheel Path	1	s of surface pavement broken loose, normally ranging in. diameter and $lac{1}{2}$ to 2 in. in depth.		
	Light	Not deemed to be a traffic hazard	Keep under observation	
	Severe	Flying debris deemed a traffic hazard	Full Depth	Figure 10.4
Pop Outs Wheel Path	Small pieces of surface pavement broken loose, normally >3" diameter and 2" in depth.			
	Light	Deemed to be a traffic hazard	Full Depth	Figure 10.4
	Severe	Flying debris deemed a traffic hazard	Full Depth	Figure 10.4
MISCELLANEOUS DISTRESS				
	Elevation di	ifferences across joints or cracks.		
Faulting	Light	Faulting <4/32"	None	
-	Moderate	4 <faulting 32"<="" <16="" td=""><td>Grind</td><td></td></faulting>	Grind	
	Severe	Faulting >16/32"	Grind	
	Light	0 <drop-off <1"<="" td=""><td>None</td><td></td></drop-off>	None	
Lane To Shoulder Drop-Off	Moderate	1" <drop-off <3"<="" td=""><td>Build Up</td><td>N/A</td></drop-off>	Build Up	N/A
,	Severe	drop-off >3 "	Build Up	
Water Bleeding Or Pumping	Seeping or ejection of water through joints or cracks.		Install appropriate drainage, edge drain, permeable subbase, reseal joints, etc.	N/A
Blowups		rement at transverse joints or cracks often d by shattering of the concrete.	Full Depth	Figure 10.3 and 10.4

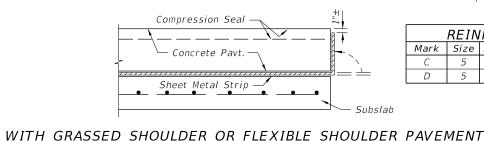
≥ DESCRIPTION:

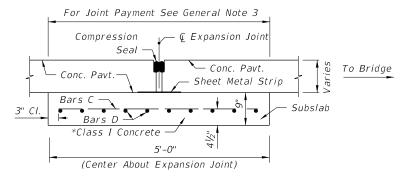
△ Expansion Joints Shall Be Constructed Parallel To The Existing Transverse Pavement Joints On Rehabilitation Projects, And Parallel To The Standard Transverse Pavement Joints Shown In The Plans For New Construction.

#### PLAN



WITH RIGID SHOULDER PAVEMENT





REINFORCING STEEL							
Mark   Size   Spac. No. Reqd.   Length							
С	5	6"	Varies	4'-6"			
D	5	6"	10	W Minus 6"			

* Finish surface smooth. Cure with heavy coating of wax base white pigmented curing compound. Apply second application immediately prior to placing pavement.

# SECTION AA

# EXPANSION JOINT

DESCRIPTION:

Immediately prior to placing the seal, the joint shall be thoroughly cleaned of all foreign material. Immediately after the seal is placed, sheet metal strip shall be bent up against the pavement edge.

The sheet metal strip shall be a minimum 16 gage steel, 12" wide and shall be galvanized in accordance with ASTM A-526, Coating Designation G90.

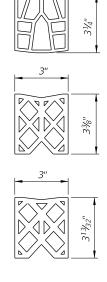
#### DETAIL SHOWING SHEET METAL STRIP



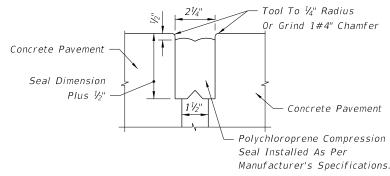
- 1. For rehabilitation projects, the designer must indicate in the plans the number of slabs to be removed, the number of subslabs to be constructed/reconstructed, and the location of expansion joints.
- Pay quantity of expansion joint to be calculated across pavement at right angles to the centerline of the roadway pavement. Shoulder pavement joint included.

#### GENERAL NOTES

- 1. The centerline of roadway and the centerline of bridge do not necessarily coincide. Prior to the placement of the expansion joint, the centerline of the roadway pavement shall be determined.
- 2. For information on other types of concrete pavement joints see Index 350-001.
- 3. Pay quantity for expansion joint is the length of joint to be constructed across the roadway and shoulder pavements, measured at right angles to the centerline of the roadway. Payment for expansion joint shall be full compensation for joint construction, including reinforced concrete subslab, sheet metal strip and compression seal, but, not including roadway pavement reconstruction associated with joint replacement or reconstruction. Expansion joint to be paid for under the contract unit price for Bridge Approach Expansion Joint, LF.



# OPTIONAL SEALS



Note: All contacting surfaces between the compression seal and concrete shall be thoroughly coated with a lubricant-adhesive.

JOINT DIMENSIONS

COMPRESSION SEAL DETAIL

LAST REVISION 11/01/17

FDOT

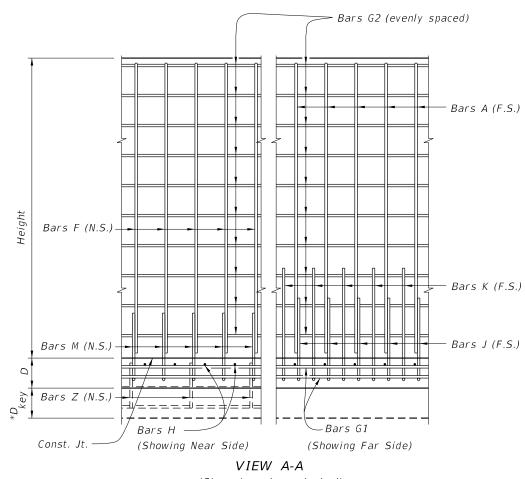
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BRIDGE APPROACH EXPANSION JOINT CONCRETE PAVEMENT

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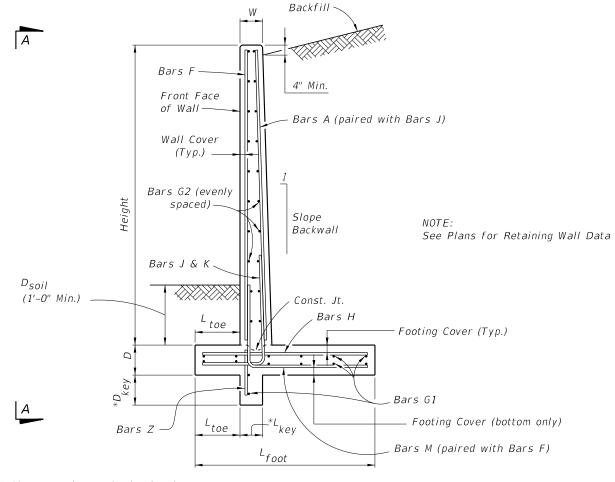
(Shear key shown dashed)

#### NOTES

TRAFFIC RAILINGS OR PARAPETS:

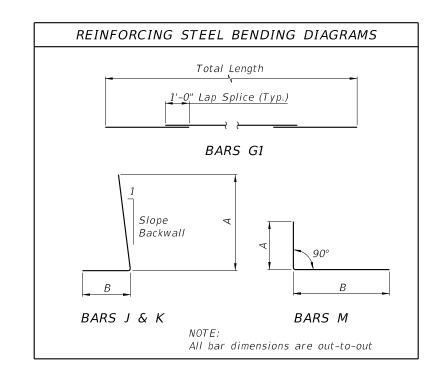
If there is a Traffic Railing or Parapet on the wall, align Wall Joints with V-Grooves, and Wall Expansion Joints with Barrier Open Joints.

FOUNDATION: Prepare the soil below the footing in accordance with the requirements for spread footings in Specification Section 455.



* Shear Key is required only when specified in the Plans.

TYPICAL SECTION



LAST **REVISION** 11/01/17

DESCRIPTION:

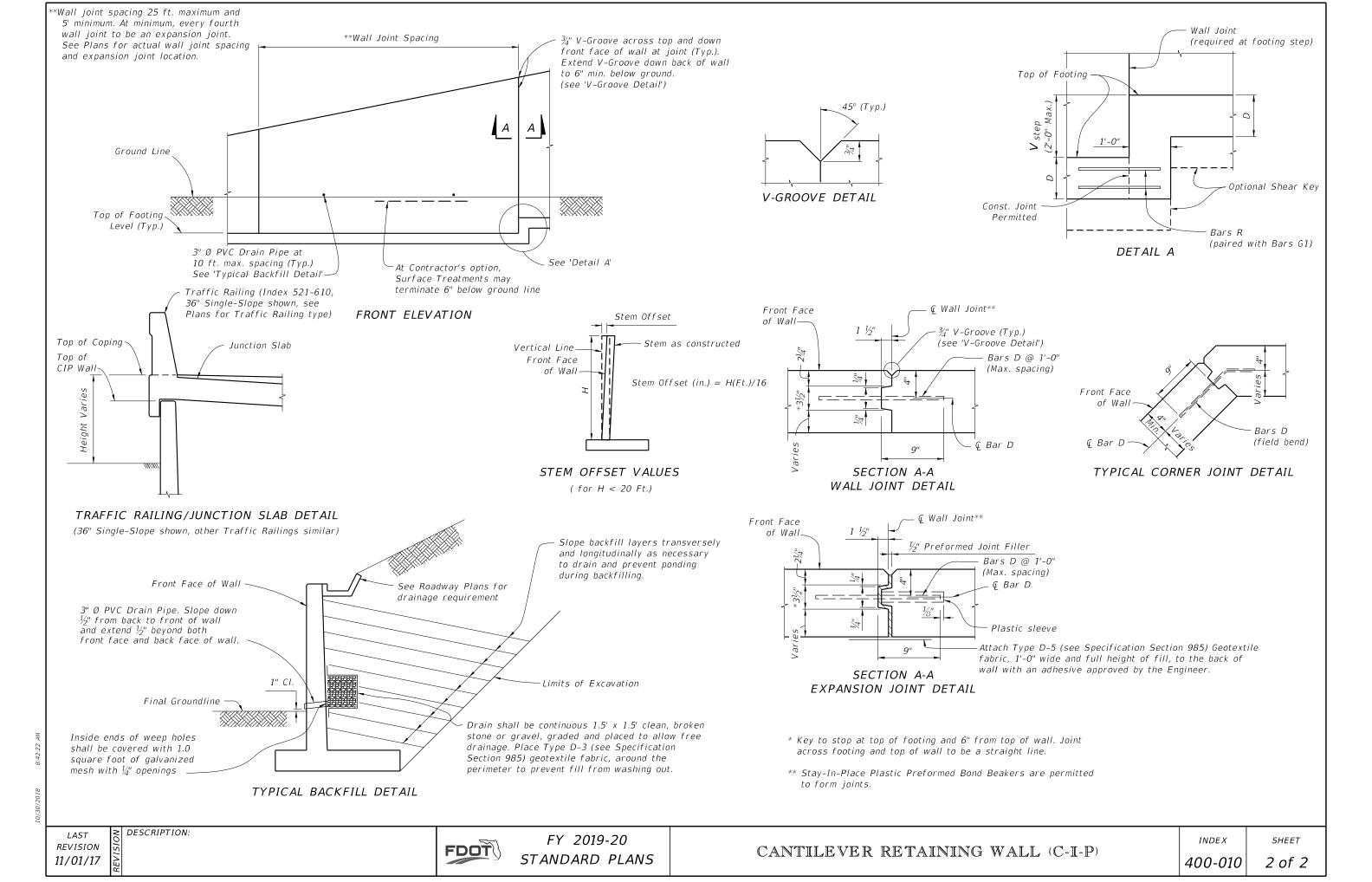
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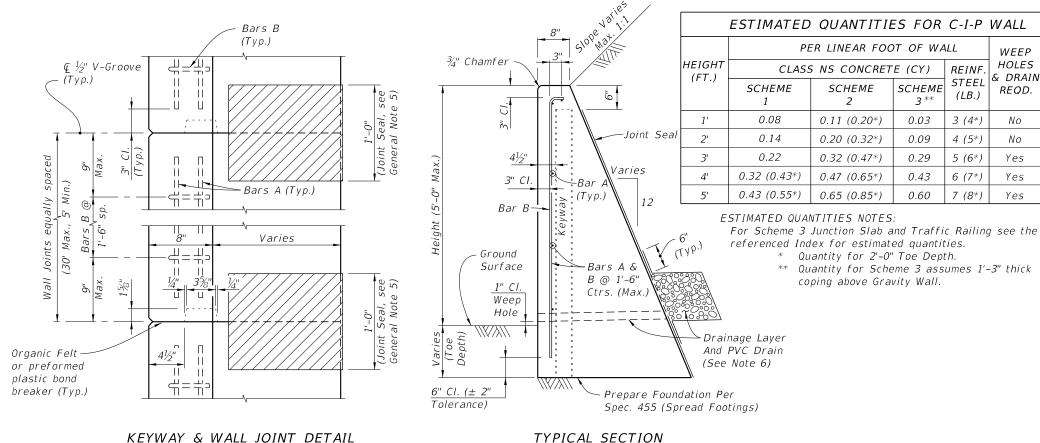
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CANTILEVER RETAINING WALL (C-I-P)

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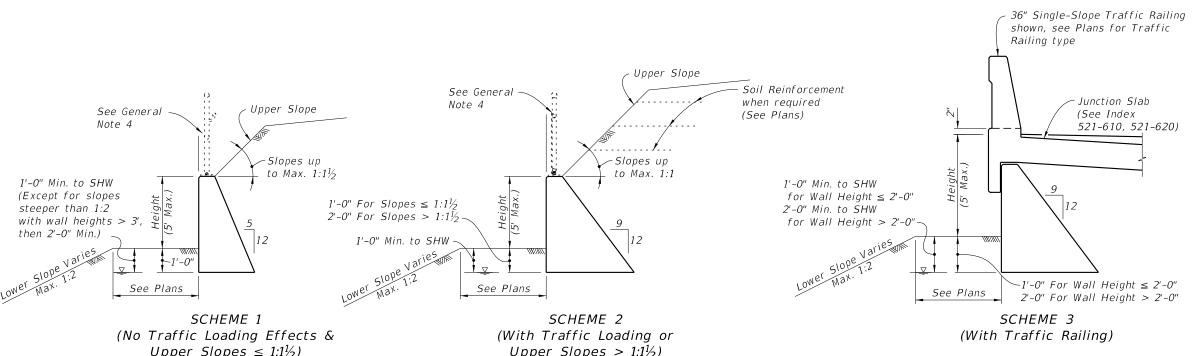


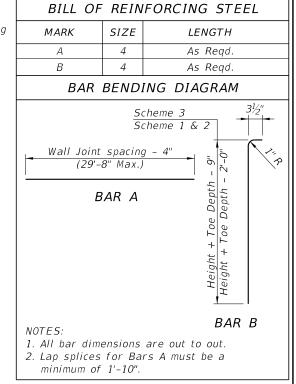
(TOP VIEW)

# TYPICAL SECTION C-I-P CONCRETE GRAVITY WALL

#### GENERAL NOTES

- 1. C-I-P Gravity Walls constructed as extensions of reinforced concrete retaining walls, except walls of proprietary designs, shall have the same face texture and finish as the reinforced concrete retaining wall.
- 2. Concrete for Gravity Wall shall be Class NS per Section 347. Concrete for Scheme 3 Junction Slab and Traffic Railing shall be Class II per Section 346, unless otherwise specified in the plans.
- 3. Reinforcing steel shall meet the requirements of Specification Section 931 (Grade 40 or 60). Smooth or Deformed Welded Wire Reinforcement (WWR) may be substituted on an equal area basis. Do not increase bar/wire spacing for Grade 60 reinforcing steel or WWR.
- When required, for adjunct guiderail, see Index 515-070 or 515-080 as appropriate. For adjunct Type B fence see Index 550-002.
- Joint Seal: Organic Felt bond breaker in accordance with Specification Section 400 or Type D-5 geotextile fabric in accordance with Specification Section 985. Mop all contact surfaces of concrete and Organic Felt or geotextile fabric with cut-back asphalt. Stop Organic Felt or geotextile fabric 6" below top of wall.
- 6. Provide a continuous 1'x1' clean gravel or crushed rock drain for wall heights 3 ft. and higher. Wrap drainage layer as shown, with Type D-3 geotextile fabric in accordance with Specification Section 985. Provide 8"x8" galvanized mesh with  $\frac{1}{4}$ " openings, at the inside end of the PVC Drain Pipe. Provide 2" Ø PVC Drain Pipe (Sch. 40) at 10 ft. max. spacing (when Drainage Layer is required). Locate outermost edge of Drain Pipe a minimum of 2'-0" from wall joints.
- 7. Cost of reinforcing steel, face texture, finish, joint seal, drain pipes, drainage layer, galvanized mesh and geotextile fabric to be included in the Contract Unit Price for Concrete Class NS, Gravity Wall. Cost of concrete for Junction Slab in Scheme 3, to be included in Contract Unit Price for Concrete Traffic Railing Barrier With Junction Slab. Adjunct railings or fences to be paid for separately.





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DESCRIPTION:

FDOT

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GRAVITY WALL

WEEP

HOLES

& DRAIN

REQD.

No

Yes

Yes

Yes

REINF.

STEEL

(LB.)

3 (4*)

4 (5*)

5 (6*)

6 (7*)

7 (8*)

SCHEME

3 **

0.03

0.09

0.29

0.43

0.60

SCHEME

2

0.11 (0.20*)

0.20 (0.32*)

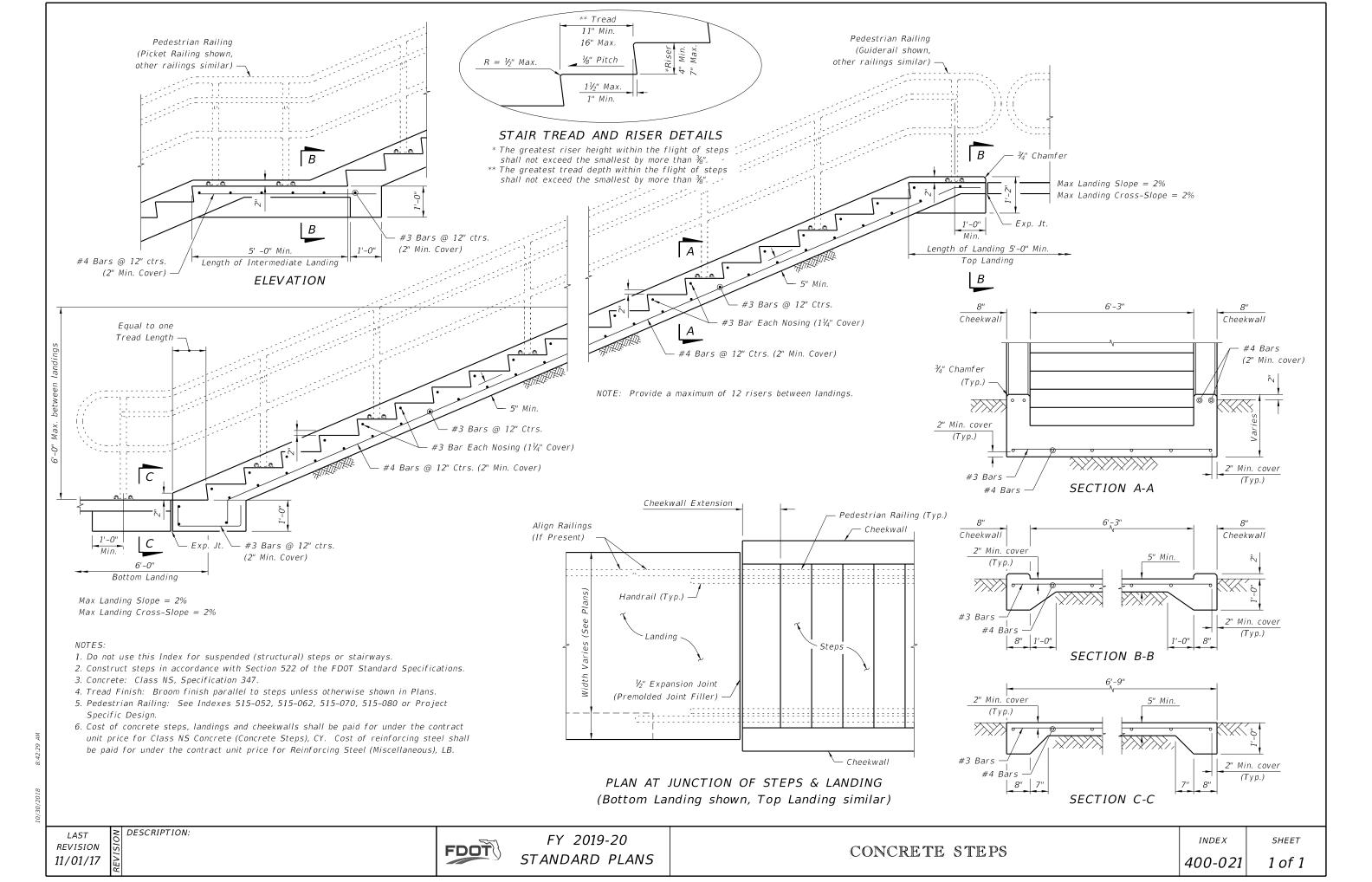
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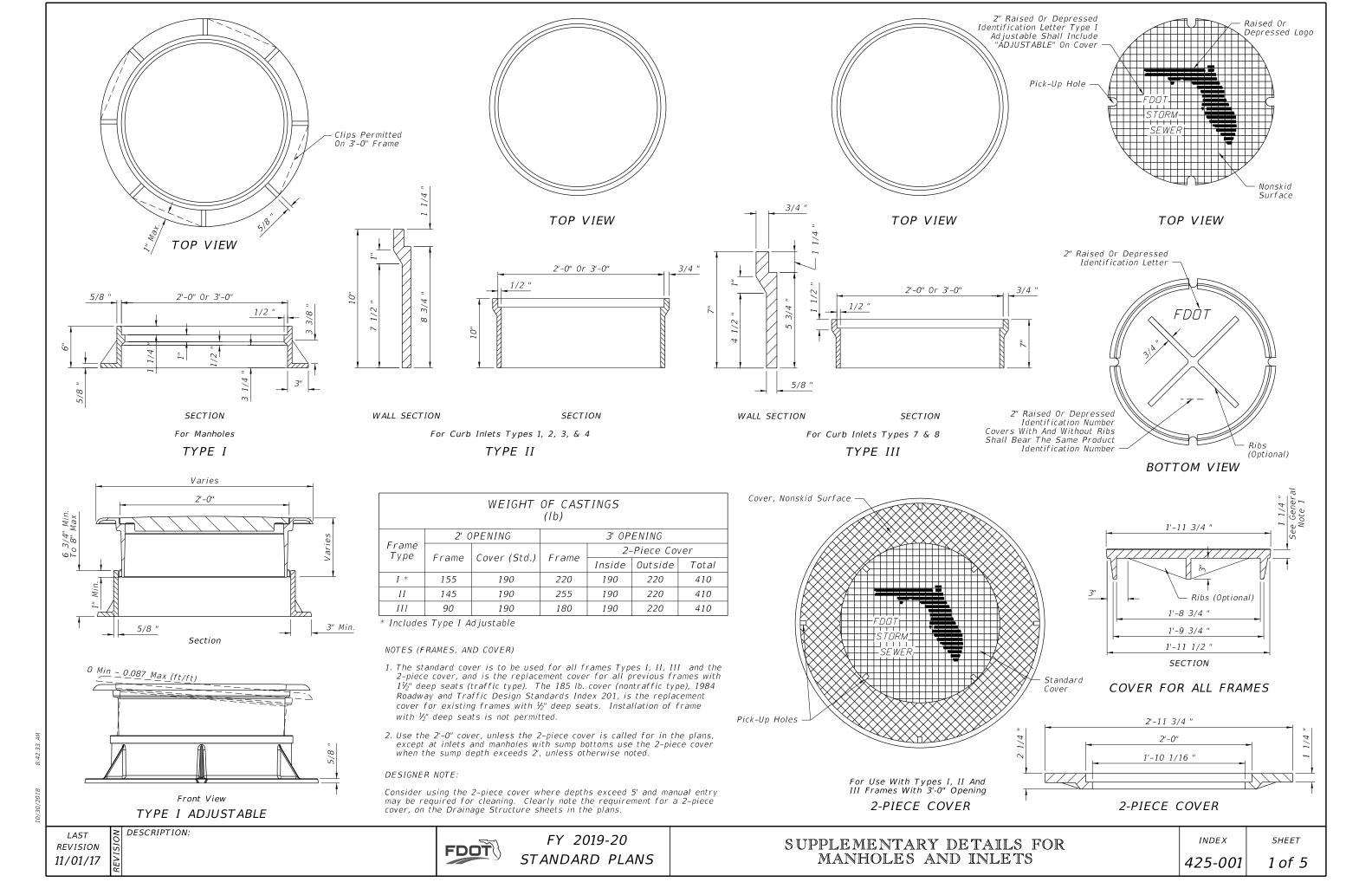
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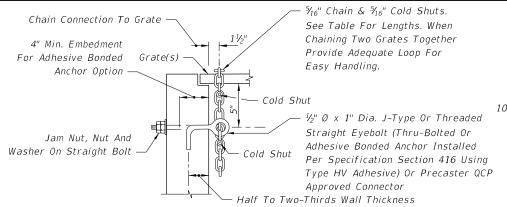
0.65 (0.85*)

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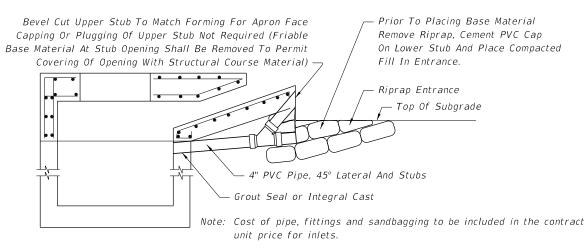


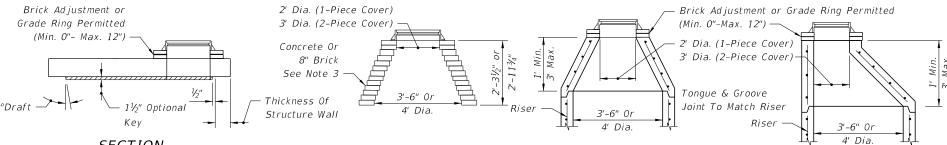
NOTE: When Alternate "G" grate is specified, the chain, bolt, nuts, washer and cold shuts shall be galvanized in accordance with Section 425 of the Standard Specifications.

Cost of eyebolt and chain to be included in the contract unit price for inlets.

	EYEBOLT AND CHAIN REQUIREMENTS						
Index Number	Inlet Type	Eye- Bolts	Length Of Chain	Handling & Remarks			
	1	1	4'-0"	Slide & Spin			
	2	1	4'-0"	Slide & Spin			
425-030	3	2	2 @ 4'-0"	Slide & Spin			
	4	2	2 @ 4'-0"	Slide & Spin			
	5	2	2 @ 4'-0"	Slide & Spin			
425-031	N/A	1	3'-8"	Slide Or Slide & Spin			
425-032	N/A	1	4'-0"	Slide & Spin			
425-040	5	1	4'-0"	Slide & Spin			
425-041	V	1	4'-0"	Slide & Spin			
425-050	Α	1	3'-0"	Slide			
425-051	В	1	5'-0"	Slide & Spin			
	С	1	2'-6"	Slide & Spin			
	D	1	2'-6"	Slide & Spin			
425-052	Ε	2	2 @ 2'-6"	Slide & Spin			
	Н	2	2 @ 2'-6"	Flip Ctr. Grate and Slide & Spin Single Free Grate			
			1 or 2 @ 1'-6"	Center Grate(s) Chained To One End Grate			
	F	1	3'-6"	Flip Or Slide & Spin			
425-053	G	1	6'-0"	Slide			
			2'-0"	Lifting Loop			
425-054	J	1	4'-0"	Slide & Spin			

#### EYEBOLT AND CHAIN FOR LOCKING GRATES TO INLETS





SECTION

Note: See Slab Designs Index 425-010.

BRICK OR CONCRETE PRECAST CONCENTRIC CONE PRECAST ECCENTRIC CONE TYPE 8

#### MANHOLE TOPS

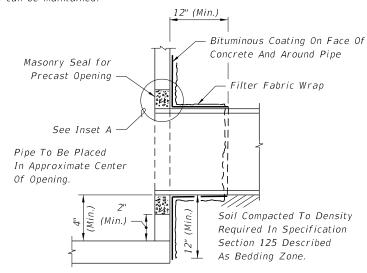
#### NOTES (TOPS)

TYPE 7

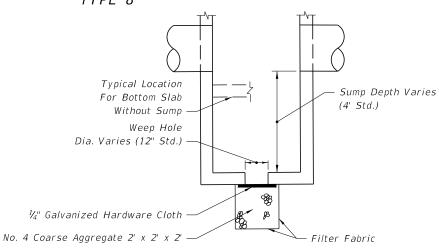
- 1. Manhole top Type 7 slabs shall be of Class II concrete. Concrete as specified in ASTM C478 may be used for precast units; see General Note 3.
- 2. Manhole top Type 7 slabs may be of cast-in-place or precast construction. The optional key is for precast tops and in lieu of dowels. Frame and slab openings are to be omitted when top is used over a junction box.
- 3. Manhole top Type 8 may be of cast-in-place or precast concrete construction or brick construction. For concrete construction, the concrete and steel reinforcement shall be the same as the supporting wall unit. An eccentric cone may be used.
- 4. Manhole tops shall be secured to structures by optional construction joints as shown on Sheet 3.
- 5. Frames can be adjusted a maximum 12" height with brick or precast ASTM C478 grade rings.
- 6. Substitution of manhole top Type 8 for manhole top Type 7 is allowed provided that minimum dimensions shown above are not reduced.
- 7. Substitution of Manhole top Type 7 for Type 8 is allowed if the minimum thickness (h) above pipe opening cannot be maintained with manhole top Type 8.

#### DESIGN NOTES

1. Manhole top Type 8 should be specified in the plans when depths shown above can be maintained.

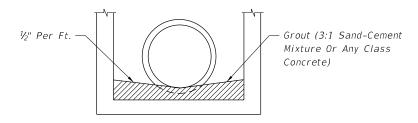


FILTER FABRIC WRAP ON GROUTED PIPE TO STRUCTURE JOINT

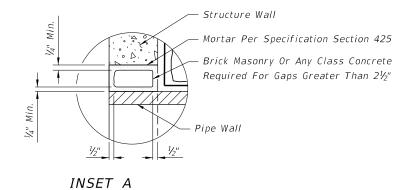


NOTE: Sump bottom appropriate for all manhole and inlet types. Sumps are to be constructed in inlet and manholes connected to French Drains unless excluded in the plans. At other locations, sump is to be constructed only where called for in the plans. Weep holes to be constructed in sump bottom only where called for in the plans. Cost of sump bottom and weep hole to be included in the contract unit price for inlet or manhole.

#### SUMP BOTTOM



# FOR ALL STRUCTURES UNLESS EXCLUDED BY SPECIAL DETAIL ALL PIPE TYPES DRAINAGE STRUCTURE INVERT



TEMPORARY DRAINS FOR SUBGRADE AND BASE

REVISION 11/01/17

DESCRIPTION:

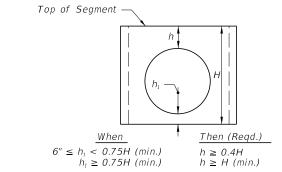
FDOT

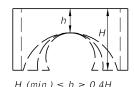
FY 2019-20 STANDARD PLANS

SUPPLEMENTARY DETAILS FOR

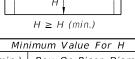
INDEX

SHEET



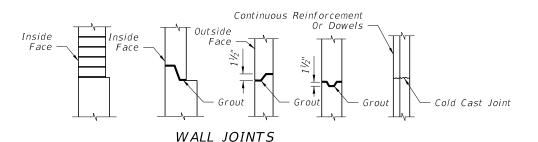


Segments may be inverted. Opening for pipe shall be the pipe OD plus 6" ( $\pm$  2" tolerance). If h can not be attained, then a top or bottom slab must be attached to the segment

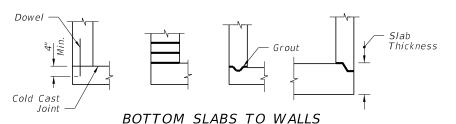


7-1111	milain Value 101 m
H (min.)	Box Or Riser Diameter
1'-0"	3'-6" & 4'-0"
1'-6"	5'-0" & 6'-0"
2'-0"	>6'-0''

# SEPARATE RISER SEGMENTS WITH CONSTRUCTION JOINTS OTHER THAN DOWEL OPTION



Fill Keyway With Grout (When Present)



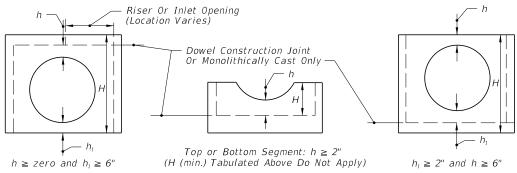
TOP SLABS TO WALLS

- 1. One or more types of joints may be used in a single structure, except brick wall structure. Brick wall construction is permitted on circular units only.
- 2. All grouted joints are to have a maximum thickness of 1".
- 3. Keyways are to be a minimum of  $1\frac{1}{2}$ " deep.

DESCRIPTION:

- 4. Joint dowels are to be #4 bars, 12" long with a minimum of 6 bars per joint approximately evenly spaced for circular structures or at maximum 12" spacing for rectangular structures. Bars may be either Adhesive Bonded Dowels in accordance with Specification Section 416, or placed approximately 6" into fresh concrete leaving the remainder to extend into the secondary cast. Welded wire reinforcement may be substituted for the dowel bar in accordance with the equivalent steel area table on Sheet 4.
- 5. Minimum cover on dowel reinforcing bars is 2" to outside face of structure.
- 6. Joints between wall segments and between wall segments and top or bottom slabs may be sealed either by preformed plastic gasket material using the procedures given in Section 430 of the Specifications or by non-shrink grout, in accordance with Section 934 of the Specifications.
- 7. Insert products approved by the Engineer may be used in lieu of dowel embedment.

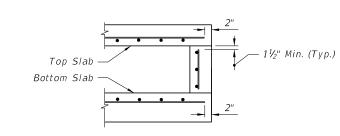
# OPTIONAL CONSTRUCTION JOINTS



# SEGMENTS FOR SLAB TO WALL DOWEL CONSTRUCTION JOINTS OR MONOLITHICALLY CAST SEGMENTS

NOTE: h may be less than 6" when approved by the Engineer, but not for inlet segments at finish grade elevation

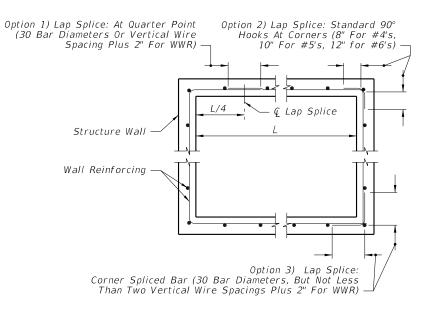
# COMPARATIVE SIDE VIEWS MINIMUM DIMENSIONS FOR BOX AND RISER SEGMENTS



as shown below.

(NOTE: NOT APPLICABLE AROUND MANHOLE AND RISER OPENINGS)

REBAR STRAIGHT END EMBEDMENT FOR TOP AND BOTTOM SLABS



# WALL REINFORCING SPLICE DETAILS

LAST REVISION 11/01/17

Cast Joint



FY 2019-20 STANDARD PLANS

SUPPLEMENTARY DETAILS FOR MANHOLES AND INLETS

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107/00/01

#### GENERAL NOTES

- 1. For square or rectangular precast drainage structures, using either deformed or smooth WWR meeting the requirements of Specification Section 931, WWR shall be continuous around the box and lapped in accordance with Option 1 or 3 as shown in the Wall Reinforcing Splice Details.
- 2. Horizontal steel in the walls of rectangular structures shall be lap spliced in accordance with Option 1, 2 or 3 as shown in the Wall Reinforcing Splice Details.
- 3. Welding of splices and laps is permitted. The requirements and restrictions placed on welding in AASHTO M259 shall apply.
- 4. Rebar straight end embedment of peripheral reinforcement may be used in lieu of ACI standard hooks for top and bottom slabs except when hooks are specifically called for in the plans or standard drawings.
- 5. Concrete as specified in ASTM C478, (4000 psi) may be used in lieu of Class II concrete in precast items manufactured in plants which meet the requirements in accordance with Specification Section 449.
- 6. Precast opening for pipe shall be the pipe 0D plus 6" (± 2" tolerance). Mortar used to seal the pipe into the opening will be of such a mix that shrinkage will not cause leakage into or out of the structure. Dry-pack mortar may be used in lieu of brick and mortar construction to seal openings less than 2½" wide.
- 7. For pay item purposes, the height used to determine if a drainage structure is greater than 10 feet shall be computed using:

  A. the elevation of the top of the manhole lid.
  - B. the grate elevation or the theoretical gutter grade elevation of an inlet, or
  - C. the outside top elevation of a junction box less the flow line elevation of the lowest pipe or to top of sump floor.

# NOTES FOR PRECAST OPTIONS AND EQUIVALENT REINFORCEMENT SUBSTITUTION

- 1. Details for optional precast inlet construction up to depths of 15' are shown on the inlet indexes.
- 2. When precast units are used in conjunction with Alt. "B" Structure Bottoms, Index 425-010, the interior dimensions of an Alt. "B" Bottom can be adjusted to reflect these inlet interior dimensions.
- 3. Concrete which meets the requirements of ASTM C478 or Class IV must be used for precast structures constructed with 6" wall or slab thickness.
- 4. Reinforcement can be either deformed bar reinforcement or welded wire reinforcement. Bar reinforcement other than 60 ksi may be used, however only two grades are recognized; Grade 40 and Grade 60. Smooth welded wire reinforcement, will be recognized as having a design strength of 65 ksi and deformed welded wire reinforcement will be recognized as having a design strength of 70 ksi. The area of reinforcement required may be adjusted in accordance with the Equivalent Steel Area Table provided. For bars and spacings not given, the steel area required can be determined by the following equations:

Grade 40 Steel Area = 
$$A_{S}40 = \frac{60}{40} \times A_{S}60$$

Smooth Welded Wire Reinforcement Steel Area =  $A_S65 = \frac{60}{65} \times A_S60$ 

Deformed Welded Wire Reinforcement Steel Area =  $A_S 70 = \frac{60}{70} \times A_S 60$ 

When a reduced area of reinforcement is provided, any maximum bar spacing shown must also be reduced as determined by the following equations, unless otherwise shown:

- Max. Grade 40 Bar Spacing = Grade 60 Bar Spacing
- Max. Smooth Welded Wire Spacing = Grade 60 Bar Spacing x 0.86
- Max. Deformed Welded Wire Spacing = Grade 60 Bar Spacing x 0.74

When an increased area of reinforcing is provided, then the maximum bar spacing may be increased by the squared ratio of increased steel area, but not to exceed 12":

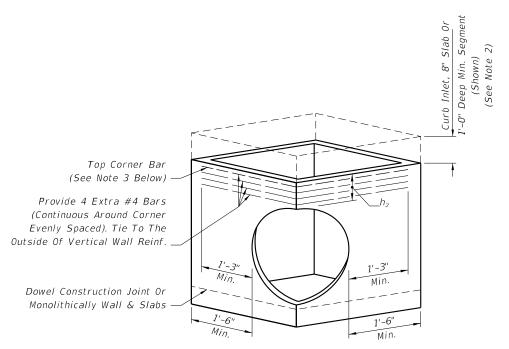
Max. Bar Spacing Provided  $\leq$  Max. Bar Spacing Required x  $\left(\frac{\text{Steel Area Provided}}{\text{Min. Steel Area Required}}\right)$ 

In no case will reinforcement with wires smaller than W3.1 or D4.0, or spacings greater than 8" be permitted. Bar reinforcement shall show the minimum yield designation grade mark or either the number 60 or one (1) grade mark line to be acceptable at the higher value. Maximum bar spacing shall not be greater than two (2) times the slab thickness with a maximum spacing of 12" or three (3) times the wall thickness, with a maximum spacing of 18" for vertical bars and 12" for horizontal bars. Wires smaller than W3.1 or D4.0 are permitted in the walls of ASTM C 478 round structure bottoms and round risers.

5. Fiber-reinforced concrete may be substituted for conventional steel reinforcement in accordance with the Structures Design Guidelines. Shop drawings corresponding to an approved fiber-reinforced concrete mix design must be submitted for approval to the State Drainage Engineer.

REVISION

11/01/17



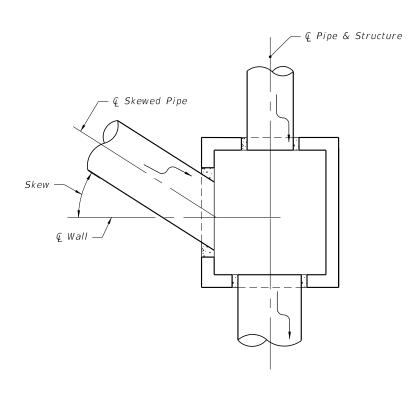
 $h_2 \ge 1'-0''$  (See Notes 2 and 3 Below)

DESIGNER NOTE: Use only when round structures are not practical, engineer of record approval required.

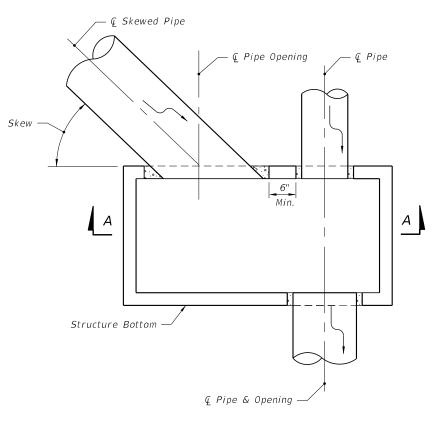
## PICTORIAL VIEW

- NOTE: 1. Submit Shop Drawings of corner openings for approval by the Engineer of Record.
  - 2.  $h_2$  may be less than 1'-0" when a minimum 1'-0" deep segment, 8" slab or curb inlet is provided above the corner opening.
  - 3. For inlet segments at finish grade elevation substitute a #8 Bar for the top corner bar when  $1'-0'' \le h_2 < 2'-0''$ .

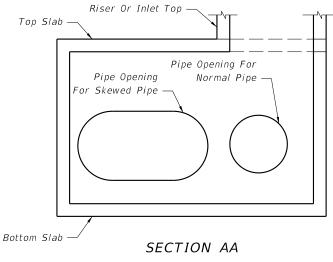
RECTANGULAR SEGMENT WITH PIPE OPENING AT CORNER



PLAN VIEW FOR SKEWS ≤ 45° (Not Centered)



# PLAN VIEW FOR SKEWS > 45° (Not Centered)



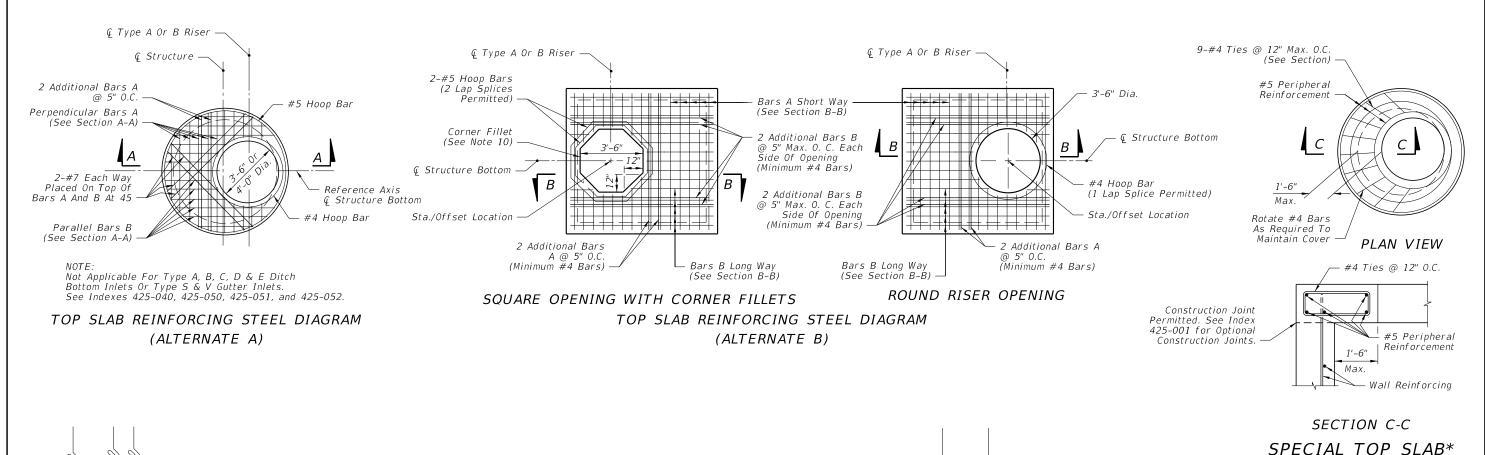
(Pipes Not Shown For Clarity)

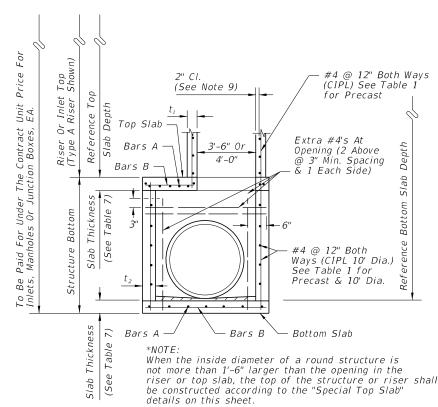
DETAILS FOR SKEWED PIPES IN RECTANGULAR STRUCTURES

**REVISION** 11/01/17

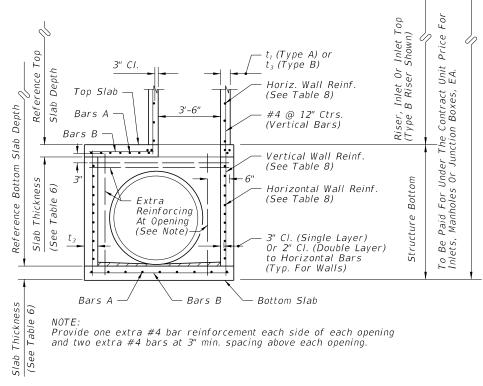
DESCRIPTION:

FDOT

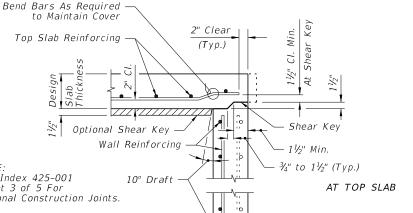


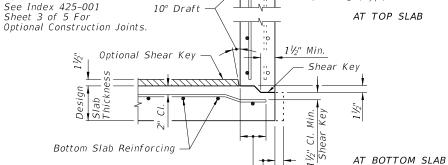






SECTION B-B (ALTERNATE B)





Rebar Straight End Embedment

4" Min. Beyond Inside Face of Structure Wall For All Bar Sizes _ Extend Top and Bottom Slabs To Achieve Minimum Rebar Embedment Beyond Inside Face

> TYPICAL SLAB TO WALL DETAILS FOR PRECAST STRUCTURES

**REVISION** 11/01/17

FDOT

FY 2019-20 STANDARD PLANS NOTF

to Maintain Cover

*INDEX* 425-010

SHEET 1 of 5

DESCRIPTION:

		Cast-	-In-Place	! Items	Precast Items				
	Structure/Riser	Clas	Class II Concrete		Clas	ss II Con	ocrete	ASTM C478	
Туре	Diameter (ft)	t ₁	t ₂	As	t ₁	t ₂	As	tı or t2	A2 ***
		Riser (in.)	Bottom (in.)	(in.²/ft.)	Riser (in.)	Bottom (in.)	(in.²/ft.)	(in.)	(in.²/ft.)
Р	3'-6"	6	8	0.20	6	8	0.20	4**	0.105
Р	4'-0''	6	8	0.20	6	8	0.20	5**	0.120
J	5'-0"	-	8	0.20	-	8	0.20	6**	0.150
J	6'-0"	1	8	0.20	-	8	0.20	6	0.180
J	7'-0"	-	8	0.20	-	8	0.20	7	0.210
J	8'-0"	ı	8	0.20	-	8	0.20	8	0.240
J	10'-0"	-	10	0.40##	-	10	0.40##	10	0.300
J	12'-0"	1	10	0.40##	_	12	0.40##	12	0.360

#### TABLE 1 NOTES:

##Provide 0.20 eq. in.2/ft. at each face, 12" max. bar spacing.

**Modified minimum wall thickness.

***Min. total circumferential reinforcement for continuous steel hoops:

 $A_2 = 0.40$  sq. in. for riser section height equal or less than 2'-0" (2 hoop min.)

 $A_2 = 0.60$  sq. in. for riser section height more than 2'-0" up to 4'-0" (3 hoop min.) Areas of reinforcing for precast items are based on Grade 60 reinforcing; No reduction in the area of reinforcement is allowed for welded wire fabric in Table 1; Area of vertical reinforcing may be reduced in accordance with ASTM C478.

SQUARE & RECTANGULAR STRUCTURES (ALTERNATE B) – TABLE 2						
Tuna	Wall Length	Max.	Wall Thick	kness (t₃)		
Туре	(ft)	Depth (ft)	CIP (in.)	Precast (in.)		
Р	≤ 3'-6"	40	6 Riser 8 Bottom	6		
J	4'-0"	40	8	6		
J	5'-0"	22	-	6		
J	6'-0"	15	-	6		
J	5'-0" to 9'-0"	40	8	8		
J	10'-0"	26	8	8		
J	10'-0" to 12'-0"	40	10	9		
J	16'-0"	35	-	9		
J	16'-0"	40	10	10		
J	20'-0"	25	-	9		
J	20'-0"	30	10	10		

TABLE 2 NOTES:

See Table 8 for Reinforcing Schedule.

#### GENERAL NOTES

- 1. Standard structure bottoms 4'-0" diameter and smaller (Alt. A) and 3'-6" square (Alt. B) are designated Type P. Larger standard structure bottoms are designated Type J. Risers are permitted for all structures. Round risers are designated Type A, square risers are designated Type B.
- 2. Walls of circular structures (Alt. A) constructed in place may be of brick or reinforced concrete. Precast and rectangular structures (Alt. B) shall be constructed of reinforced concrete only.
- 3. Wall thickness and reinforcement are for either reinforced cast-in-place or precast concrete units except that precast circular units may be furnished with walls in accordance with ASTM C478 (see modified wall thicknesses in Table 1).
- 4. Top and bottom slab thickness and reinforcement are for precast and cast-in-place construction. All concrete shall be of Class II concrete, except use Class IV concrete when shown in the Plans, for special applications of structures located in extremely aggressive environments. Concrete as specified in ASTM C478 (4000 psi) may be used in lieu of Class II concrete for precast items manufactured in accordance with Specifications Section 449.
- 5. All reinforcement shown is Grade 60 steel, deformed bar. Equivalent area Grade 40 steel or equivalent area smooth or deformed welded wire reinforcement in accordance with Specification Section 931 may be substituted according to Index 425-001, unless otherwise noted.
- 6. Alt. A or Alt. B structure bottoms may be used in conjunction with curb inlet tops Types 1, 2, 3, 4, 5, 6, 9, and 10, and any manhole or junction box unless otherwise shown in the plans or other standard drawings. Alt. B structure bottoms may be used in conjunction with curb inlet Types 7 & 8, or any ditch bottom inlet unless otherwise shown in the plans or other standard drawings.
- 7. Rectangular structures may be rotated as directed by the Engineer in order to facilitate connections between the structure walls and storm sewer pipes.
- 8. Except when ACI hooks are specifically required, reinforcement in top and bottom slab shall be straight embedment.
- 9. All reinforcement must have 2" minimum cover except for 3'-6" diameter precast circular units manufactured under ASTM C478, keyed construction otherwise shown. Additional bars used to restrain hole formers for precast structures with grouted pipe connections, may be left flush with the hole surface. Cut or bend reinforcement at pipe openings to maintain cover. Exposed ends of reinforcing at precast pipe openings and grouted joints must be removed to 1" below the concrete surface and sealed with a Type F epoxy in accordance with Specification Section 926. Horizontal steel in rectangular structures shall be lapped a minimum of 30 bar diameters or by standard hooks at corners.
- 10. The corner fillets shown are necessary for rectangular structures used with circular risers and inlet throats and when used on skew with rectangular risers, inlets and inlet throats. Fillets will be required in the top slab of the Alt. A structure bottoms when used with the Alt. B risers. Each fillet shall be reinforced with two #5 bars.
- 11. Inlet walls, throats, risers or manhole tops shall be secured to structures as shown on Index 425-001 Optional Construction Joints.
- 12. Structures with depths over 14' below the mean high water table are to be checked for flotation by the designer of the drainage project.
- 13. Units larger than specified standards may be substituted at the contractor's option when these units will not cause or increase the severity of utility conflicts. Such larger units shall be furnished at no additional cost to the Department. Larger Alt. A units cannot replace Alt. B units without approval of the Engineer. This note applies to this Index only.
- 14. For manhole and junction box tops, for frames and covers, and, for supplementary details and notes see Index 425-001.
- 15. Type J structure bottoms must have a minimum 6'-0" wall height when possible, for maintenance access.

PER SIDE						
	RECTANG	ULAR	ROUND			
PIPE	Side Dimens	sion (L)	Diamet	er (D)		
SIZE	Single Pipe	Note	Single Pipe	2 to 4		
3122	Per Side	Number	or	Pipes		
	Tel Side	Number	θ=180°	θ=90°		
18"	3'-6"		3'-6"	4'-0"		
24"	3'-6"		3'-6"	5'-0"		
30"	3'-6"/4'-0"	2	4'-0"	6'-0"		
36"	4'-0"/5'-0"	3	5'-0"	7'-0"		
42"	5'-0"		6'-0"	7'-0"		
48"	6'-0"		6'-0"	8'-0"		
54"	6'-0"		7'-0"	10'-0"		
60"	7'-0"		7'-0"	10'-0"		
66"	7'-0"/8'-0"	4	8'-0"	12'-0"		
7 <i>2</i> "	8'-0"		8'-0"	12'-0"		
78"	9'-0"		10'-0"	12'-0"		
84"	9'-0"		12'-0"	N/A		

#### TABLE 3 NOTES:

- For Round Structures sizes with variable angles between pipes and variable pipe sizes, refer to the FDOT Storm Drain Handbook.
- 2. For 3'-6" Precast Square Structure Bottoms, 30" Pipes with similar invert elevations are not permitted in adjacent walls. Use 4'-0" Side Dimensions when 30" pipe openings are required on adjacent walls and the difference in flow lines is less than 3'-0".
- 3. For 4'-0" Precast Square Structure Bottoms, 36" Pipes with similar invert elevations are not permitted in adjacent walls. Use 5'-0" Side Dimensions when 36" pipe openings are required on adjacent walls and the difference in flow lines is less than 3'-0".
- 4. For 7'-0" Precast Square Structure Bottoms, 66" Pipes with similar invert elevations are not permitted in adjacent walls. Use 8'-0" Side Dimensions when 66" pipe openings are required on adjacent walls and the difference in flow lines is less than 4'-0".

TABLE 4-MINIMUM SIZES FOR MULTIPLE									
PARALLEL PIPE CONNECTIONS FOR									
REC	RECTANGULAR STRUCTURE BOTTOMS								
PIPE	PIPE	MINIMUM	WALL LENGTH	H (L) FOR					
SIZE	SPACING	NUMBE	R OF PARALLEI	_ PIPES					
312.5	(S)	2	3	4					
18"	2'-10"	6'-0"	8'-6"	11'-0"					
24"	3'-5"	6'-6"	10'-0"	13'-6"					
30"	4'-3"	8'-0"	12'-6"	16'-6"					
36"	5'-1'	9'-6"	14'-6"	19'-6"					
42"	6'-0"	11'-0"	17'-0"	-					
48"	6'-9"	12'-6"	19'-0"	-					
54"	7'-8"	14'-0"	-	-					
60"	8'-6"	15'-0"	-	-					
66"	9'-0"	16'-6"	-	-					
72"	10'-0"	18'-0"	_	-					
78"	10'-9"	19'-0"	_	-					
84"	11'-8"	20'-6"	-	_					

#### TABLE 4 NOTES:

- 1. Minimum wall lengths based on precast structures, using concrete pipe with maximum skew angles per Table 5.
- 2. Wall lengths exceeding 20'-0" require special designs.

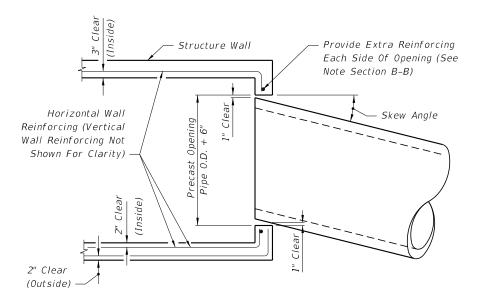
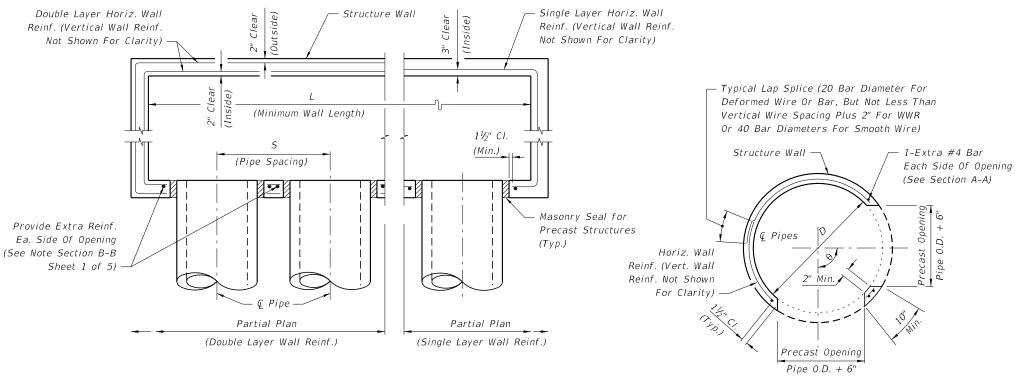


TABLE 5 - MAXIMUM PIPE SKEW FOR													
	PRECAST ROUND OPENINGS												
	WALL		PIPE SIZE										
	THICKNESS	18"	18" 24" 30" 36" 42" 48" 54" 60" 66" 72" 78" 84"					84"					
MAXIMUM	8"	19° 17° 16° 16° 15° 14° 14° 13° 13° 13° 12° 12°						12°					
SKEW ANGLE	6"	21°	20°	18°	17°	17°	16°	15°	15°	$14^{\circ}$	14°	13°	13°

TABLE 5 NOTES:

These values are based on 2" clearance for precast structures. Larger skews are possible for Cast-In-Place Structures or elliptical pipe openings when approved by the Engineer.

# MAXIMUM PIPE SKEW FOR PRECAST ROUND OPENINGS PLAN VIEW



MULTIPLE PARALLEL PIPE CONNECTIONS DETAIL
PLAN VIEW

PRECAST ROUND STRUCTURES WITH MULTIPLE PIPE CONNECTIONS

# STRUCTURE SIZES FOR PIPE CONNECTIONS

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

STRUCTURE BOTTOMS TYPE J AND P

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425-010

# SLAB DESIGNS - SQUARE AND RECTANGULAR STRUCTURES (TABLE 6) (ALL SLABS 8" THICK EXCEPT AS NOTED - REINFORCING PARALLEL TO SHORT WAY AND LONG WAY)

SHORT	Γ-WAY	LONG-WAY						
SLAB DEPTH	SCHEDULE (Bars A)	SLAB DEPTH	SCHEDULE (Bars B)					
SIZE: 3'-6" x UNLIMITED								
≥0.5' < 8'	B10	≥0.5' < 24'	B10					
8' < 13'	B5.5	24'-40'	B5.5					
13' < 31'	C6.5							
31'-40'	D7							
	CIZE: 1' v	UNLIMITED						
>0 El - 7!	B5.5	≥0.5' < 15'	D10					
≥0.5' < 7' 7' < 19'	C6.5	≥0.5 < 15 15' < 29'	B10 B5.5					
19' < 31'	D7	29'-40'	C6.5					
31'-40'	E5	29-40	60.5					
31 40								
	SIZE:	5' x 5'						
≥0.5' < 3'	C6.5	≥0.5′ < 3′	C6.5					
3' < 7'	B5.5	3' < 13'	C6.5					
7' < 22'	C6.5	13' < 22'	D7					
22' < 29'	D7	22' < 29'	D4.5					
29'-40'	E5	29'-40'	E5					
	SIZE:	5' x 6'						
≥0.5′ < 12′	C6.5	≥0.5′ < 3′	C6.5					
12' < 26'	D7	3' < 9'	B5.5					
26'-40'	E5	9' < 23'	C3.5					
		23' < 35'	D4.5					
		35'-40'	E5					
	SIZE:	5' x 7'						
≥0.5′ < 10′	C6.5	≥0.5′ < 10′	B5.5					
10' < 20'	D7	10' < 31'	C3.5					
20' < 34'	E5	31'-40'	D4.5					
34'-40'	F5							
	CI7E,	5' x 8'						
≥0.5′ < 7′	C6.5	≥0.5′ < 8′	B10					
7' < 13'	D7	8' < 17'	B5.5					
13' < 24'	E5	17' < 25'	C6.5					
24'-40'	F5	25'-40'	C3.5					
	SIZE:	5' x 9'						
≥0.5' < 8'	C6.5	≥0.5' < 14'	B10					
8' < 14'	D7	14' < 24'	B5.5					
14' < 25'	E5	24' < 34'	C6.5					
25'-40'	F5	34'-40'	C3.5					
0.51		UNLIMITED						
≥0.5' < 8'	C6.5	≥0.5' < 14'	B10					
8' < 14'	D7	14' < 24'	B5.5					
14' < 25'	E5	24' < 34'	C6.5					
25'-40'	F5	34'-40'	C3.5					

1	LONG-WAY			
SCHEDULE (Bars A)	SLAB DEPTH	SCHEDULE (Bars B)		
SIZE:	6' x 6'			
C6.5	≥0.5' < 10'	C3.5		
		D4.5		
		E5		
		E3		
	33'-40'	F 5		
SIZE:	6' x 7'			
C6.5	≥0.5′ < 8′	C6.5		
D7	8' < 12'	C3.5		
E5	12' < 21'	D4.5		
F5	21' < 28'	E5		
	28' < 35'	E3		
	35'-40'	F 5		
SIZE:	6' x 8'			
C6.5	≥0.5′ < 6′	B5.5		
D7	6' < 11'	C6.5		
E5	11' < 17'	C3.5		
	17' < 22'	D4.5		
		E5		
		E3		
SIZE:				
D7	>0.5' < 8'	B5.5		
		C6.5		
		C3.5		
		D4.5		
- 55		E5		
	25 5 7			
SIZE: 6' x	UNLIMITED			
D7	≥0.5′ < 8′	B5.5		
E5	8' < 14'	C6.5		
F5	14' < 21'	C3.5		
G5	21' < 25'	D4.5		
	25'-34'	E5		
6175	7, 7,			
		C6.5		
		C3.5		
		D4.5		
F 5		E3		
		F 3.5		
		G3.5		
		C6.5		
D7		C3.5		
		D4.5		
F5	13' < 22'	E3		
G5	22' < 30'	F3.5		
	30'-40'	G3.5		
		C6.5		
		C3.5		
	10' < 14'	D4.5		
	14' < 21'	E5		
G5	21' < 29'	E5		
	(Bars A)   SIZE: C6.5   D7   E5   F5   G5   C6.5   D7   E5   F5   G5   C6.5   C7   E5   C6.5   C7   C7   C7   C7   C7   C7   C7   C	$(Bars \ A)$ $DEPTH$ $SIZE: \ 6' \ x \ 6'$ $C6.5$ $≥0.5' < 10'$ $D7$ $10' < 18'$ $E5$ $18' < 27'$ $27' < 33'$ $33'-40'$ $SIZE: \ 6' \ x \ 7'$ $C6.5$ $≥0.5' < 8'$ $D7$ $8' < 12'$ $E5$ $12' < 21'$ $E5$ $12' < 21'$ $E5$ $12' < 21'$ $E5$ $12' < 35'$ $35'-40'$ $SIZE: \ 6' \ x \ 8'$ $C6.5$ $≥0.5' < 6'$ $D7$ $6' < 11'$ $E5$ $11' < 17'$ $F5$ $17' < 22'$ $G5$ $22' < 32'$ $32'-40'$ $SIZE: \ 6' \ x \ 9'$ $D7$ $≥0.5' < 8'$ $E5$ $8' < 14'$ $F5$ $14' < 21'$ $G5$ $21' < 25'$ $25'-34'$ $SIZE: \ 7' \ x \ 7'$ $C6.5$ $≥0.5' < 4'$ $D7$ $20.5' < 8'$ $E5$ $8' < 14'$ $F5$ $14' < 21'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$ $14'$		

SHOR	T-WAY	LONG-WAY		
SLAB DEPTH	SCHEDULE (Bars A)	SLAB DEPTH	SCHEDULE (Bars B)	
		8' x 8'	(Burs b)	
0.51 1.01			5.4.5	
≥0.5′ < 10′	D7	≥0.5′ < 9′	D4.5	
10' < 19'	E5	9' < 13'	E5	
19'-30'	F 5	13' < 18'	F5	
		18' < 23'	F3.5	
		23'-30'	G3.5	
	SIZE:	8' x 9'		
≥0.5' < 8'	D7	≥0.5' < 7'	D7	
8' < 14'	E5	7' < 9'	D4.5	
14' < 23'	F5	9' < 15'	E3	
23'-31'	G3.5	15' < 20'	F5	
		20' < 23'	F3.5	
		23'-31'	G3.5	
	SIZE:	9' x 9'		
≥0.5′ < 8′	D7	≥0.5′ < 7′	D4	
8' < 14'	E5	7' < 10'	E5	
14' < 22'	F5	10' < 17'	F3.5	
		17' < 22'	G3.5	
SIZ	ZE: 9'x9'x10"	SLAB THICKN	IESS	
22' < 36'	F5	22' < 31'	F3.5	
36'-40'	G5	31'-40'	G3.5	
		SLAB THICK		
≥0.5′ < 7′	C6.5	0.5' < 6'	C6.5	
7' < 10'	D7	6' < 9'	D4.5	
10' < 18'	E5	9' < 15'	E5	
18' < 27'	F 5	15' < 22'	F5	
27'-32'	G5	22'-32'	G3.5	
	E: 12'x12'x12"	SLAB THICK		
≥0.5′ < 10′	D7	≥0.5′ < 8′	D7	
10' < 16'	E5	8' < 14'	E5	
16' < 25'	F5	14' < 22'	F5	
25'-35'	G5	22' < 30'	G5	
		30'-35'	H4	

# SLAB AND WALL DESIGN TABLE NOTES

- 1. Size is the inside dimension(s) of a structure.
- 2. Slab reinforcement is appropriate for top, intermediate, and bottom slabs.
- 3. Bottom Slabs for precast 3'-6" x 3'-6" rectangular structures at 15' depth or less, may be 6" thick.
- 4. Slab depth is measured from finished grade to top of slab.
- 5. Wall depth is measured to the top of the bottom slab for boxes and to the top of the intermediate slab for risers.
- 6. Wall height is the distance between top of lower slab to bottom of upper slab. Maximum wall height is 12' for wall lengths exceeding 5', or 10' for wall lengths exceeding 12'.

# SLAB DESIGNS - ROUND STRUCTURES (TABLE 7)

SLAB DEPTH	SLAB THICKNESS	REINF. (2-WAY) SCHEDULE									
SIZE: 3'-6" DIAMETER											
2'-15'	6" Precast	C6.5									
0.5' < 30'	8"	A6									
30'-40'	8"	B5.5									
SIZ	SIZE: 4'-0" DIAMETER										
≥0.5′ < 19′	8"	A6									
19' < 30'	8"	B5.5									
30'-40'	8"	C6.5									
SIZ	E: 5'-0" DIAMET	TER									
≥0.5′ < 15′	8"	B5.5									
15' < 26'	8"	C6.5									
26' < 35'	8"	D7									
35'-40'	8"	D4.5									
	E: 6'-0" DIAMET										
≥0.5′ < 9′	8"	B5.5									
9' < 15'	8"	C6.5									
15' < 22'	8"	C3.5									
22' < 30'	8"	D4.5									
30'-40'	8"	E5									
	E: 7'-0" DIAMET										
≥0.5' < 8'	8"	C3.5									
8' < 16'	8"	D4.5									
16' < 23'	8"	E5									
23' < 27'	8"	E3									
27'-40'	8"	F3.5									
	E: 8'-0" DIAMET										
≥0.5′ < 10′	8"	D4.5									
10' < 16'	8"	E5									
16' < 19'	8"	E3									
19' < 29'	8"	F3.5									
29'-40'	<u>10"</u> E: 10'-0" DIAME	F5									
≥0.5' < 12'	10"	D4.5									
12' < 20'	10"	E5									
20' < 28'	10"	F5									
28'-40'	10"	G3.5									
	: 12'-0" DIAME										
≥0.5' < 8'	10"	D4.5									
8' < 13'	10"	E5									
13' < 18'	10"	F5									
18' < 26'	10"	G3.5									
26'-40'	12"	G3.5									

- 7. Wall lengths exceeding 6'-0" require two layers of reinforcing (See Table 8) with 2" of cover from the horizontal bars to the inside and outside faces for each layer.
- 8. Wall lengths exceeding the dimensions or depths shown in Table 8, or 12'-0" diameter require a special design.
- 9. Wall thickness and reinforcing for rectangular structures is based on the longer wall length.
- 10. Reinforcing schedules with larger areas of steel may be substituted for schedules with smaller bar or wire spacing, except that Schedule B10 may not be substituted for Schedule A6. See Index 425-001 for allowable bar spacing adjustments when larger areas of reinforcing are substituted.

**REVISION** 11/01/17





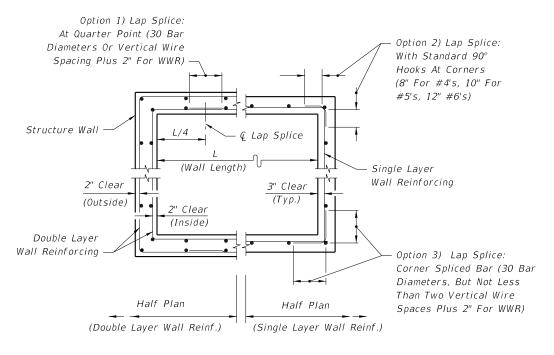
# WALL DESIGNS - RECTANGULAR STRUCTURES (TABLE 8)

VERTICAL REINFORCING			HORIZONTAL REINFORCING			WALL HICKNESS	
WALL DEPTH	SCHL	SCHEDULE WALL SCHEDULE				ΗL	
		SIZE: 3			 RS		
≥1.17' - 40'		12	≥1.17′			10	6"/8"
			10' <	18'	В	5.5	6"/8"
			18' <	29'	С	6.5	6"/8"
			29' -	40'	С.	3.5	6"/8"
		51	ZE: 4'-	-0"			
≥1.17' - 40'	Α	12	≥1.17'	< 6'	В	10	6"/8"
			6' <	10'	В	5.5	6"/8"
			10' <	20'	C	6.5	6"/8"
			20' <		С.	3.5	6"/8"
			28' -		D	4.5	6"/8"
		Si	ZE: 5'-	-0"			
≥1.17' - 40'	Α	12	≥1.17′		В	5.5	6"/8"
			5' <			6.5	6"/8"
			9' <			3.5	6"/8"
			15' <			4.5	6"/8"
			22' -		E	<b>3</b>	8"
			!ZE: 6'-				
≥1.17' < 26'	<i>A</i>	12	≥1.17′		C3.5		6"/8"
			9' <		D4.5 E3 Inside Outside		6"/8"
	Incido	Outsida	15' <	26.			8"
26' - 40'	A12	Outside A12	26' -	10'	D7	D7	8"
20 - 40	AIZ		<u>  20 -</u>  ZE: 7'-		D)	DI	0
	Incido	Outside		-0	Incido	Outside	
≥1.17' < 25'	A12	A12	≥1.17′	- 7'	B10	B10	8"
26' - 40'	B10	B10	7' <		B5.5	B5.5	8"
20 ,0	2.0	5.0	10' <		C6.5	C6.5	8"
			20' <		D7	D7	8"
			30' -	40'	E5	E5	8"
		Si	ZE: 8'-	-0"			
	Inside	Outside			Inside	Outside	
≥1.17' < 20'	A12	A12	≥1.17′	< 6'	B5.5	B5.5	8"
20' - 40'	C6.5	C6.5	6' <	13'	C6.5	C6.5	8"
			13' <	22'	D7	D7	8"
			22' <	31'	E5	E5	8"
			31' -		F5	F5	8"
		51	ZE: 9'-	-0"			
	Inside	Outside				Outside	
≥1.17' < 12'	A12	A12	≥1.17′		C6.5	C6.5	8"
12' < 28'	C6.5	C6.5	8' <		D7	D7	8"
28' - 40'	D7	D7	15' <		E5	E5	8"
			23' -		F5	F5	8"
	_		ZE: 10'	-0"	_		
·		Outside				Outside	
≥1.17' < 10'	B10	B10	≥1.17′		D7	D7	8"
10' < 21'	C6.5	C6.5	10' <		E5	E5	8"
21' < 26'	D7	D7	17' <		F5	F5	8"
26' - 40'	C6.5	C6.5	26' -	4U'	F5	F 5	10"

VERTICAL REINFORCING			HORIZONTAL REINFORCING			WALL ICKNESS				
WALL DEPTH	SCHE	EDULE	WALL DEPTH	SCHE	EDULE	ΗL				
	SIZE: 10'-0" (Precast Only)									
	Inside	Outside		Inside	Outside					
26' - 40'	D7	D7	26' - 40'	F5	F5	9"				
		SI	ZE: 12'-0"							
	Inside	Outside		Inside	Outside					
≥1.17' < 14'	B10	B10	≥1.17′ < 10′	C6.5	C6.5	10"				
14' < 25'	C6.5	C6.5	10' < 17'	D7	D7	10"				
25' - 40'	D7	D7	17' < 24'	E5	E5	10"				
			24' - 40'	F5	F5	10"				
	SIZ	ZE: 12'-	-0" (Precast	Only)						
		Outside			Outside					
≥1.17' < 12'		B10	≥1.17' < 10'	D7	D7	9"				
12' < 24'	C6.5	C6.5	10' < 17'	D4.5	D4.5	9"				
24' - 40'	D7	D7	17' < 23'	E5	E5	9"				
			23' < 32'	F5	F5	9"				
			32' - 40'	G5	G5	9"				
		SI	ZE: 16'-0"							
		Outside			Outside					
≥1.17' < 11'	C6.5	C6.5	≥1.17' < 13'	D7	D7	10"				
11' < 20'	D7	D7	13' < 20'	E5	E5	10"				
20' < 28'	E5	E5	20' < 28'	F5	F5	10"				
28' - 40'	F 5	F5	28' - 40'	G5	G5	10"				
			-0" (Precast	Only)						
		Outside			Outside					
≥1.17' < 10'	C6.5	C6.5	≥1.17' < 9'	D7	D7	9"				
10' < 18'	D7	D7	9' < 13'	D4.5	D4.5	9"				
18' < 25'	E5	E5	13' < 19'	E5	E5	9"				
25' - 35'	F5	F5	19' < 27'	F5	F5	9" 9"				
		C.I.	27' - 35'	G5	G5	9"				
			ZE: 20'-0"							
1 17/ 10/		Outside			Outside	1.01				
≥1.17' < 10'	C6.5	C6.5	≥1.1/' < 8'	D/	D/	10"				
10' < 17'	D7	D7	8' < 12'	E5	E5	10"				
17' - 30'	E5	E5	12' < 20' 20' - 30'	F 5 G 5	F 5 G 5	10" 10"				
	C1.	7 E + 20'	-0" (Precast		0.5	10				
-		Outside			Outside					
≥1.17' < 8'	C6.5	C6.5	≥1.17' < 8'	D4.5	D4.5	9"				
8' < 13'	D7	D7	8' < 12'	E5	E5	9"				
13' - 25'	E5	E5	12' < 19'	F5	F5	9"				
15 25			19' - 25'	G5	G5	9"				
			1 15 25		0,5					

REINFORCING SCHEDULE							
GRADE 60 BARS OR 65 KSI & 70 KSI WELDED WIRE REINFORCING							
	GRADE 60	MAX	XIMUM SPA	CING			
SCHEDULE	AREA	GR 60	WWR EQU	IV. AREA*			
	(in.²/ft.)	BARS (in.)	65 KSI (in.)	70 KSI (in.)			
A12	0.20	12	8	8			
A6	0.20	6	5	41/2			
B10	0.24	10	8	7½			
B5.5	0.24	5½	5	4			
C6.5	0.37	$6\frac{1}{2}$	6	5			
C3.5	0.37	3½	3	$2\frac{1}{2}$			
D7	0.53	7	6	5			
D4.5	0.53	$4\frac{1}{2}$	4	3½			
E5	0.73	5	4	4			
E3	0.73	3	3	3			
F5	1.06	5	4	4			
F3.5	1.06	3½	3	3			
G5	1.45	5	4	4			
G.3.5	1.45	3½	3	3			
H4	1.75	4	3	3			

^{*}Equivalent Area Welded Wire Reinforcing may be substituted in accordance with Index 425-001.



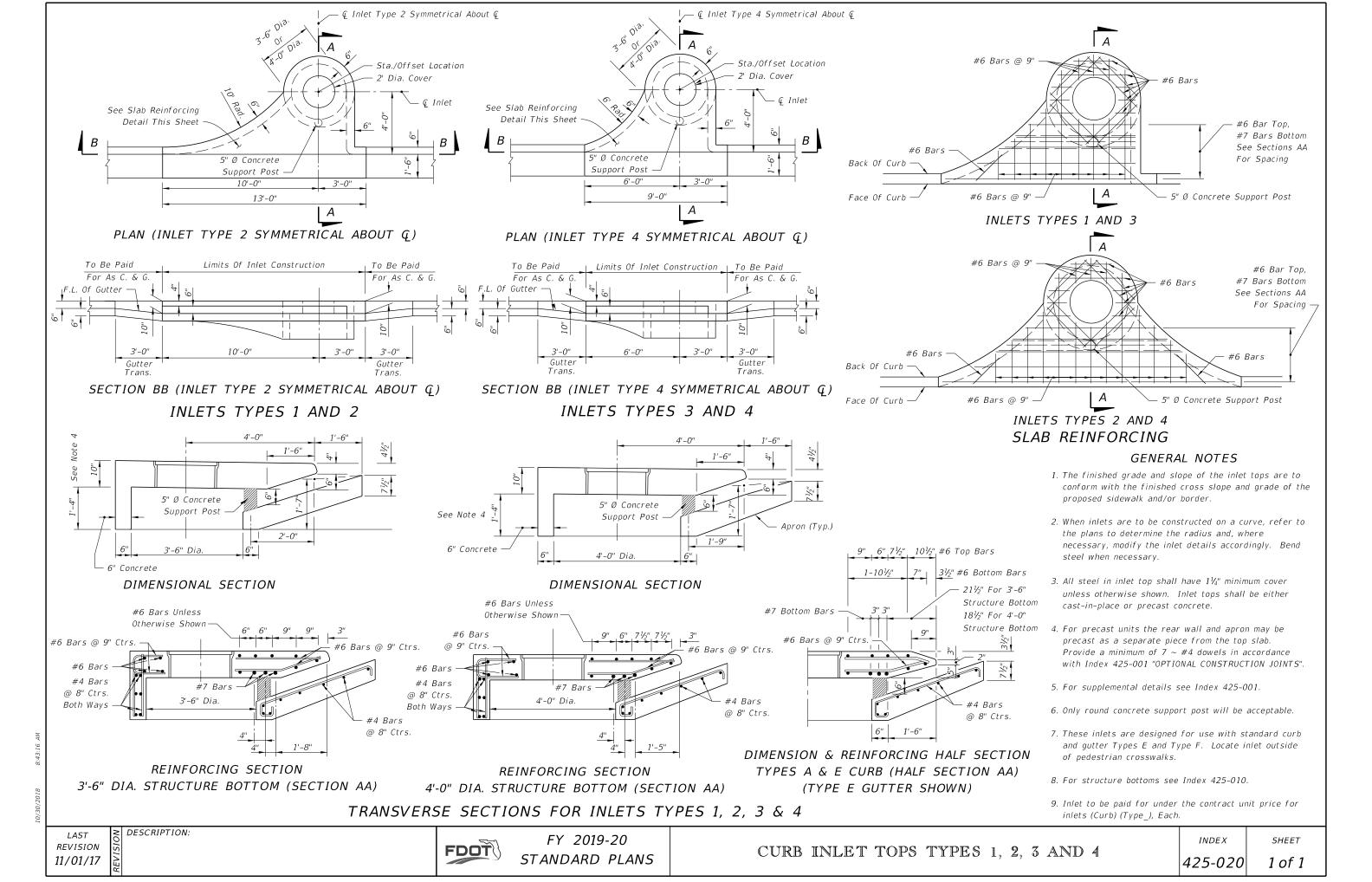
WALL REINFORCING SPLICE DETAILS (ALTERNATE B)

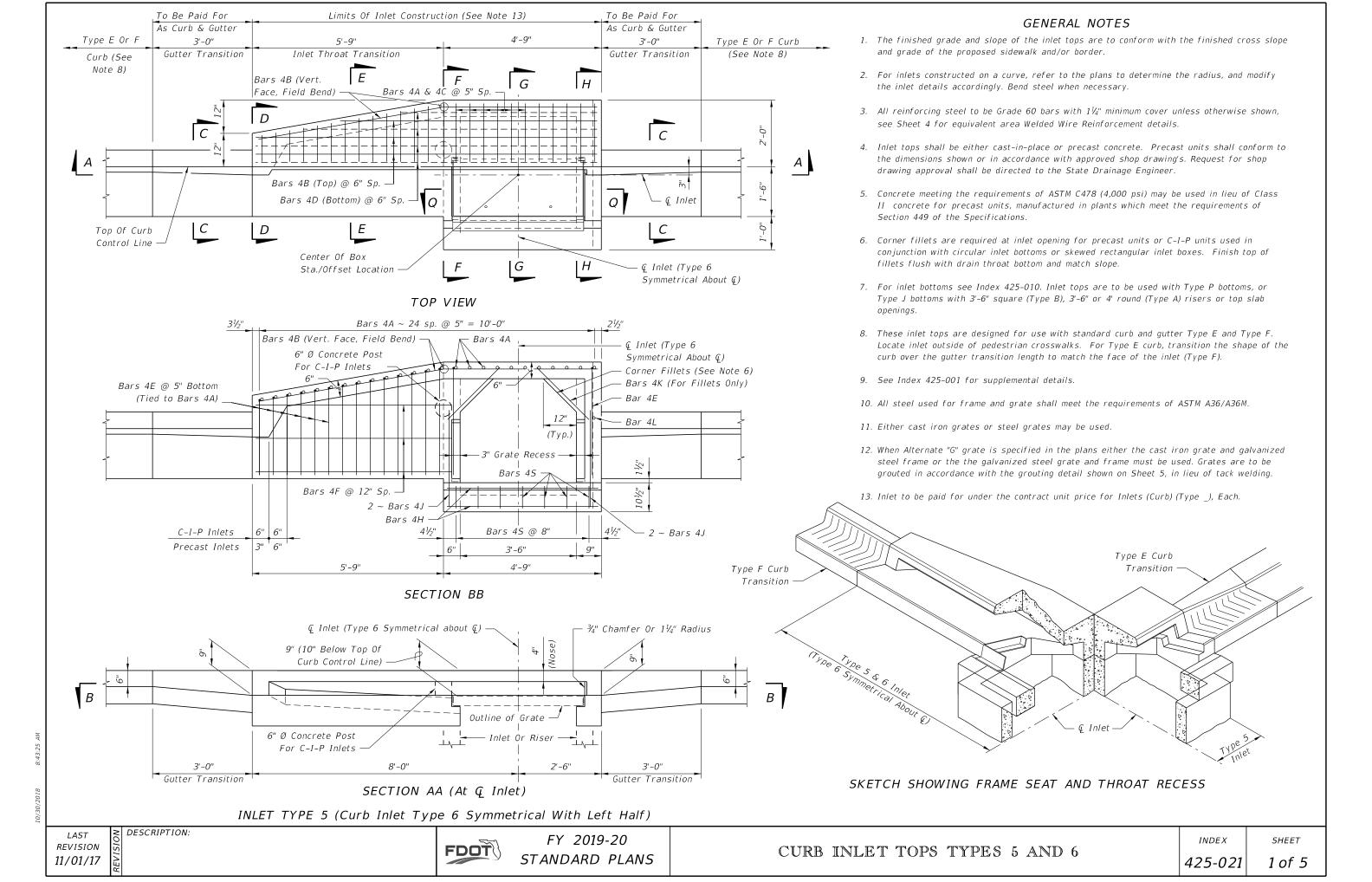
REVISION 11/01/17

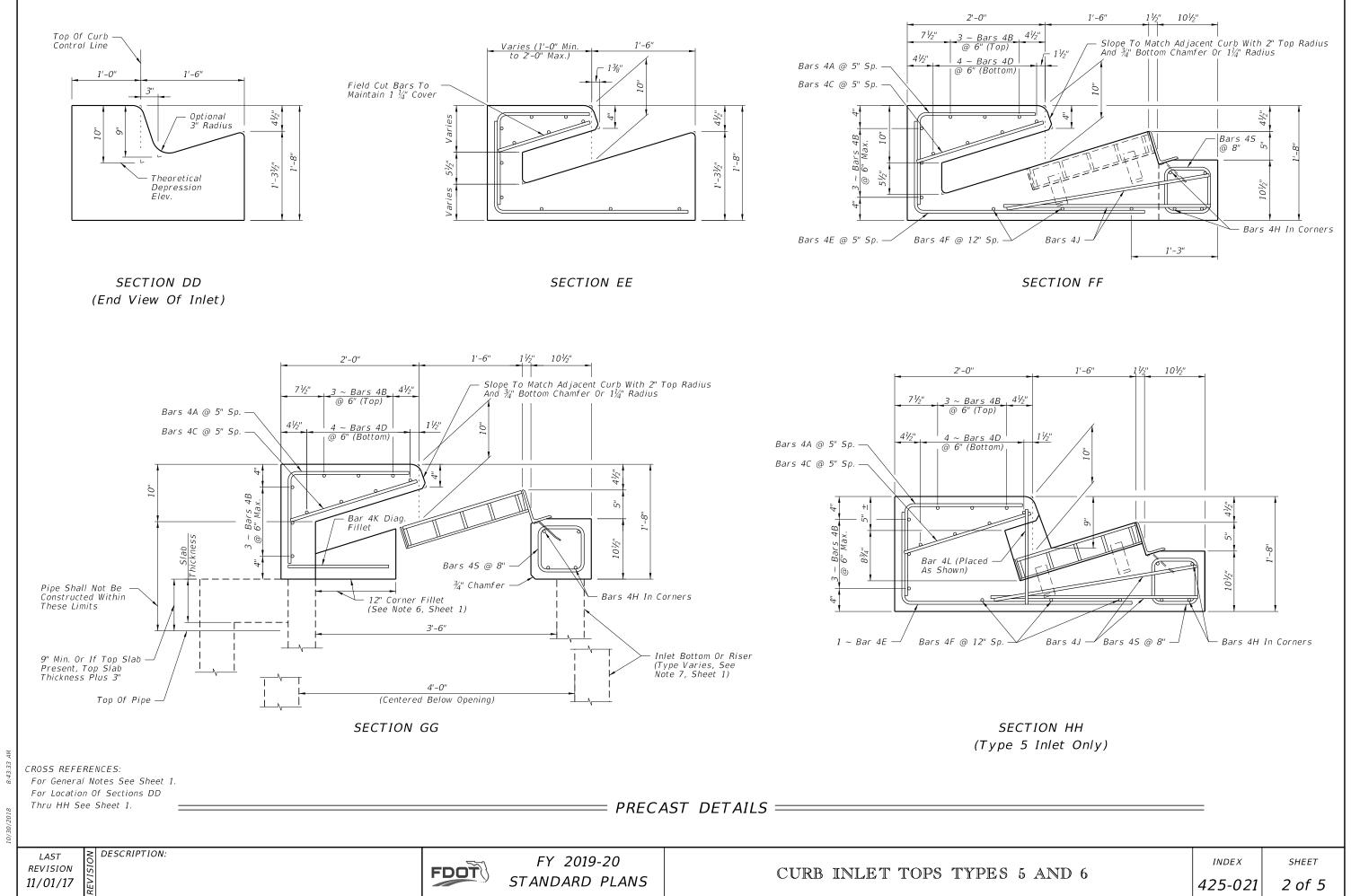
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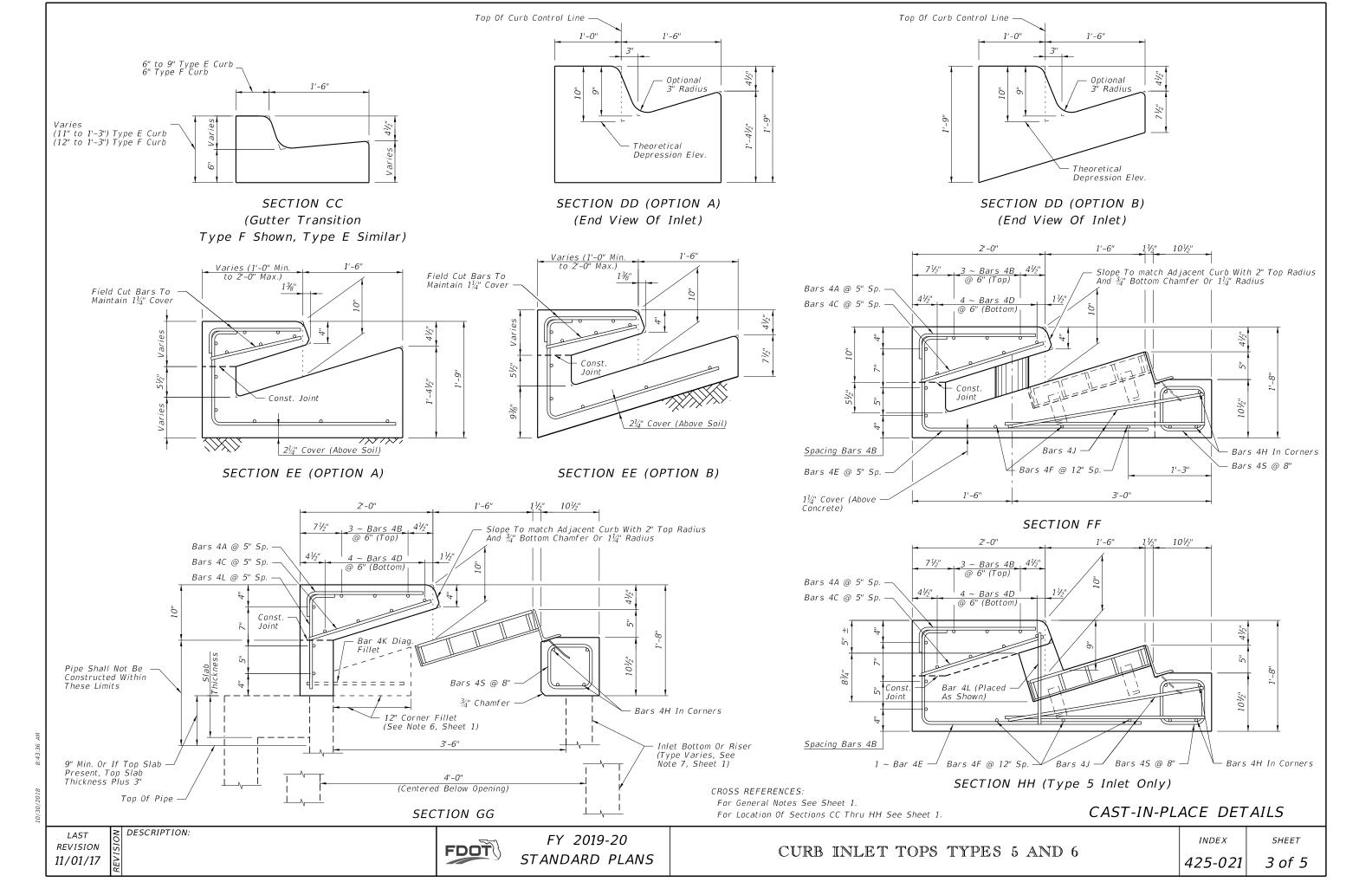
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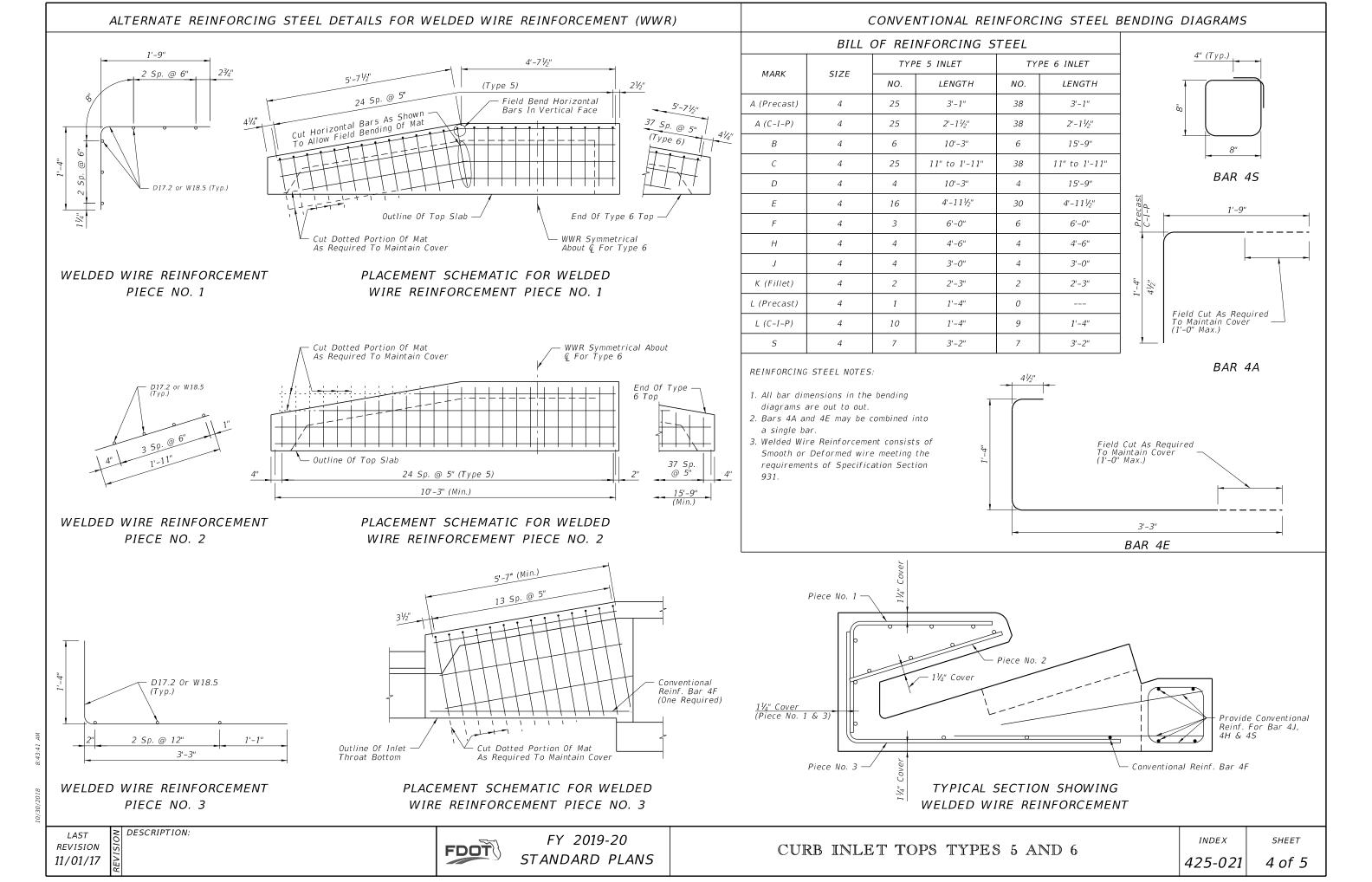
FY 2019-20 STANDARD PLANS

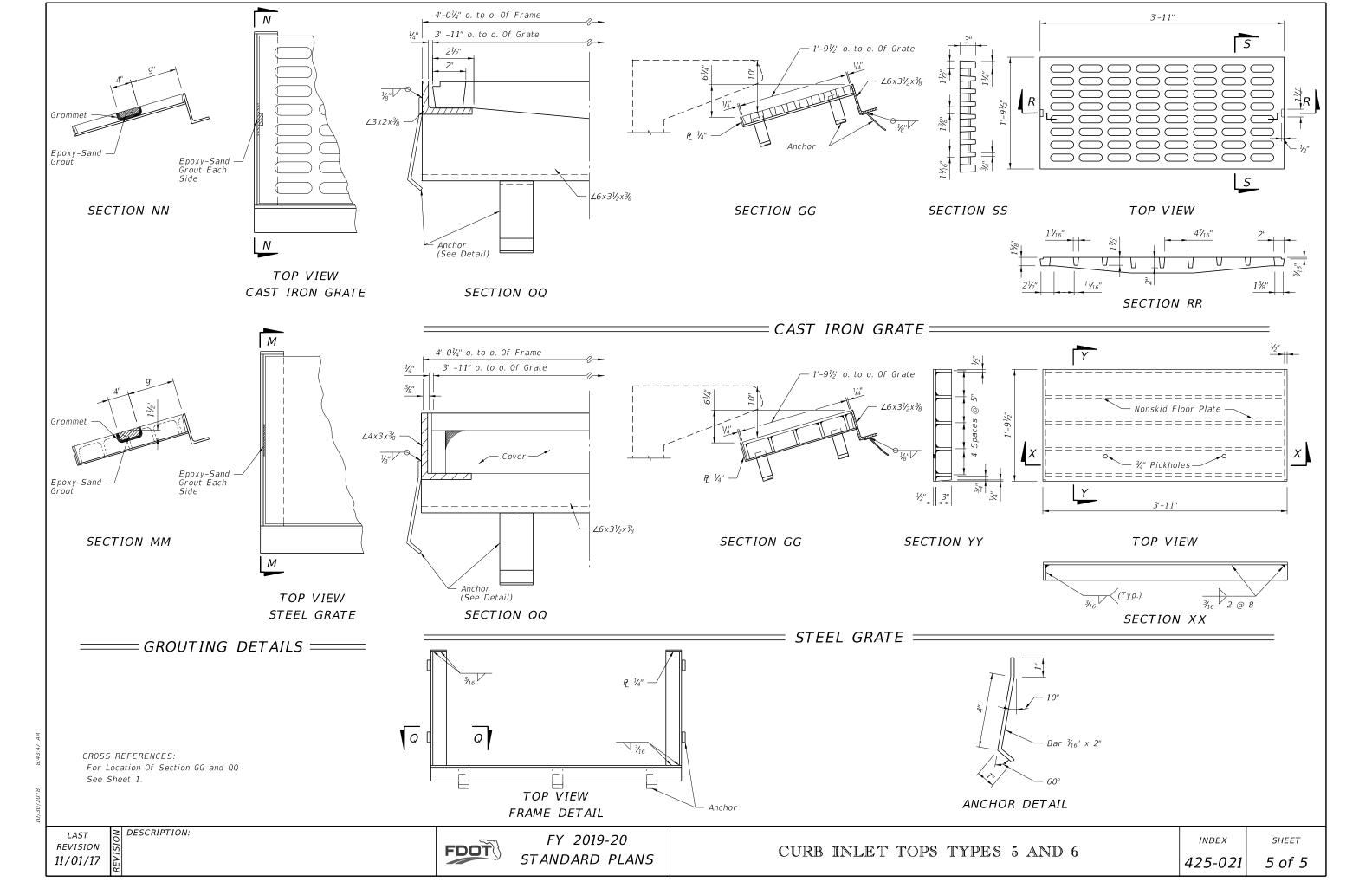


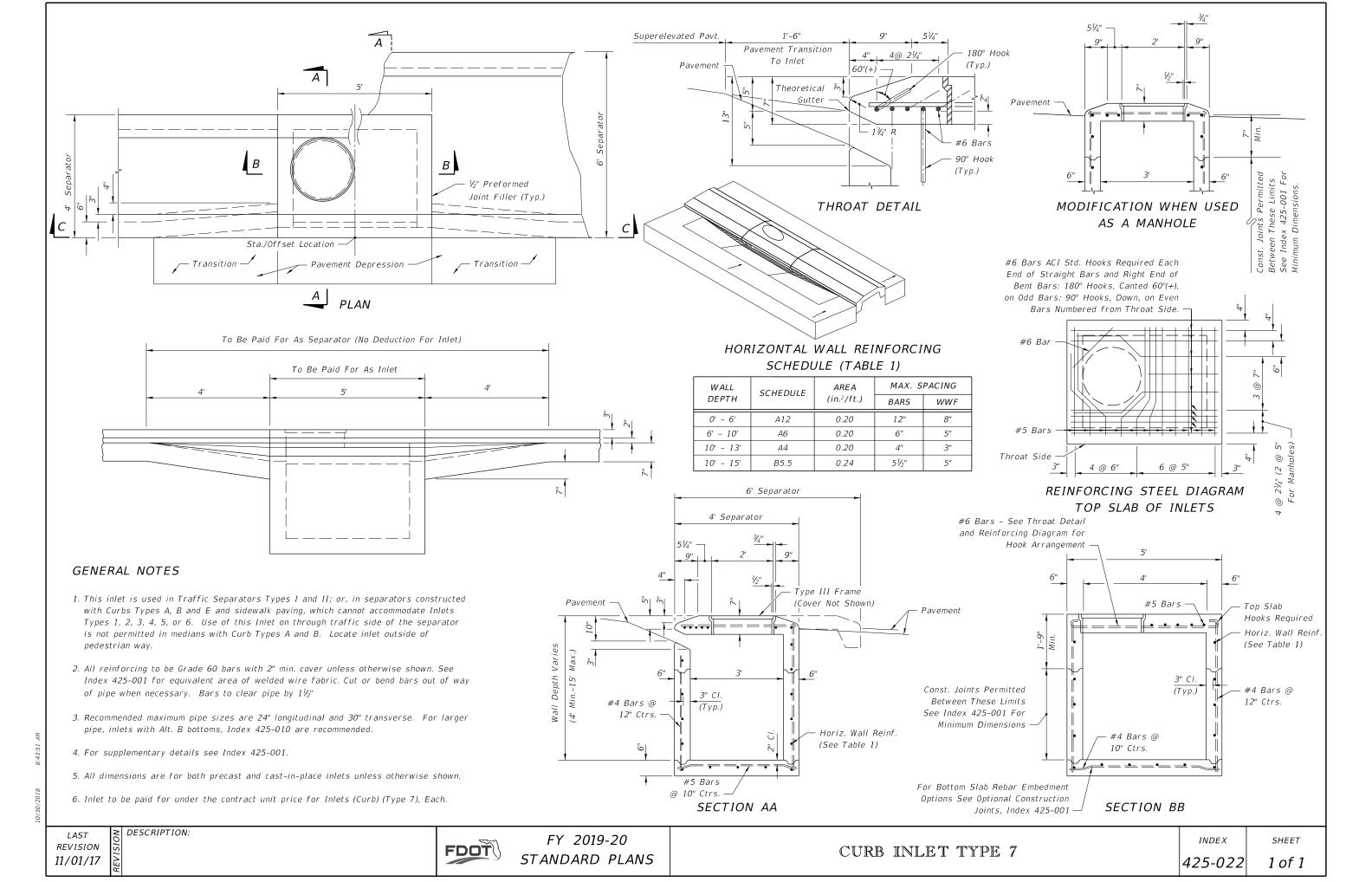


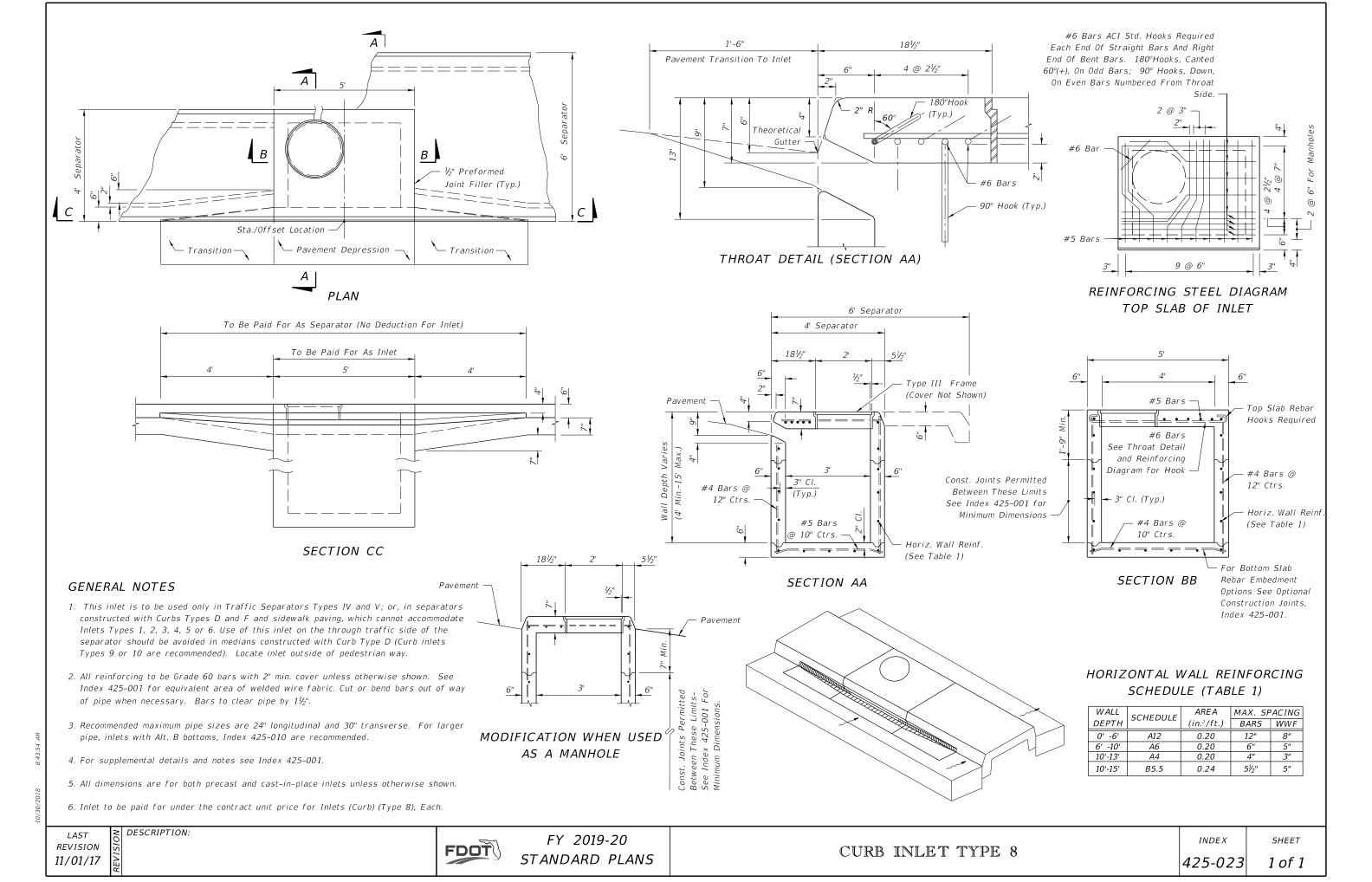


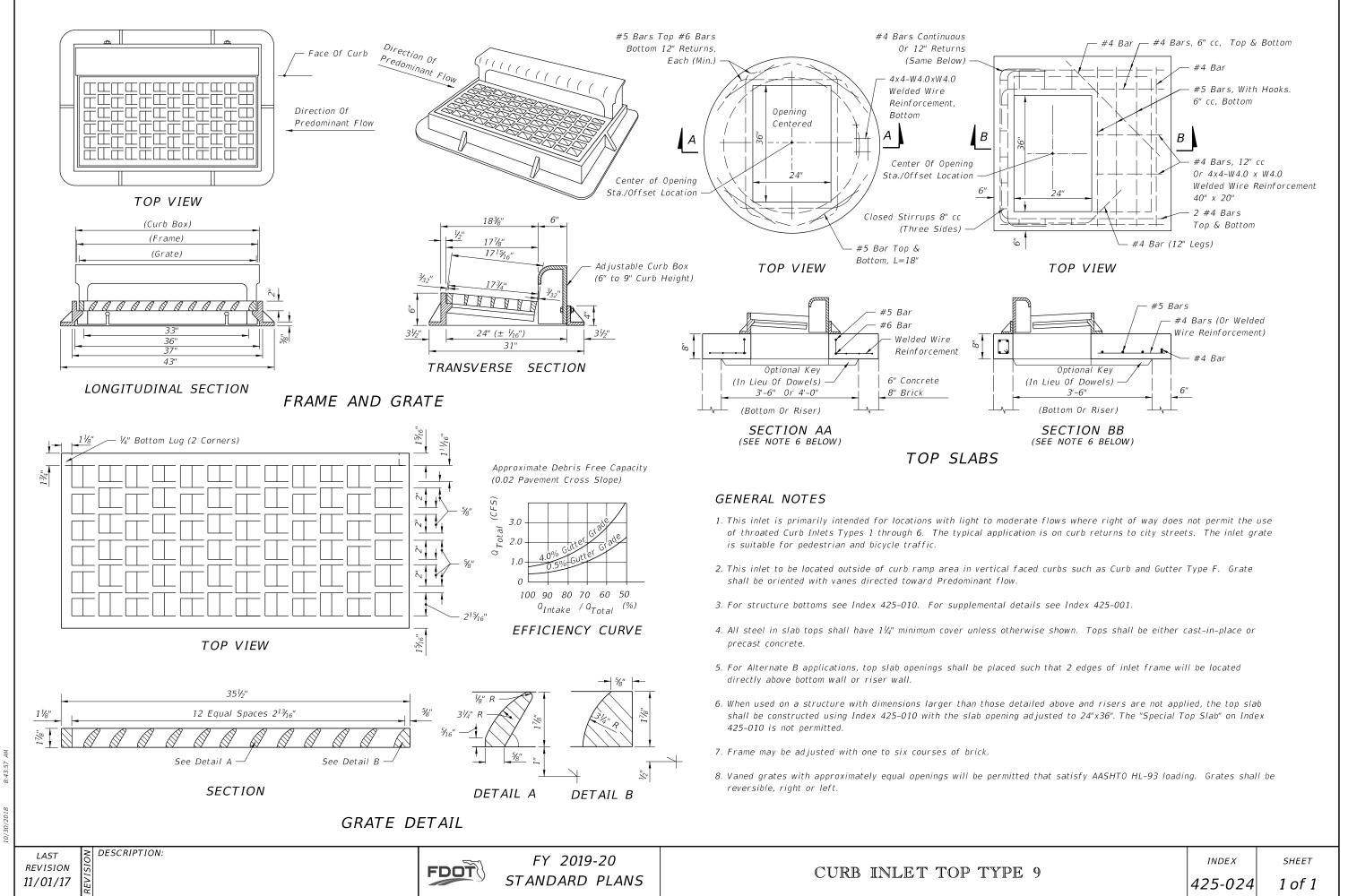


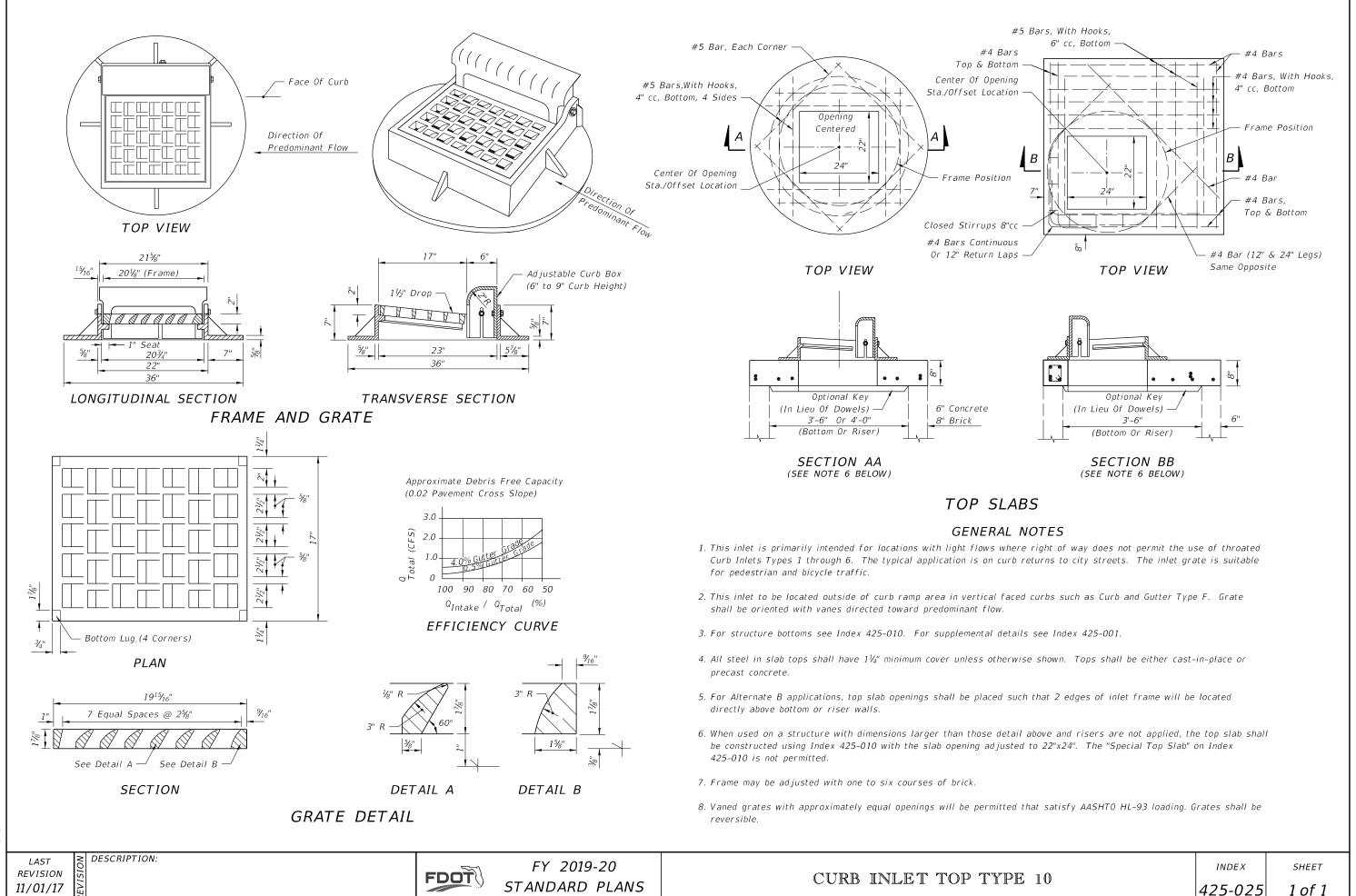




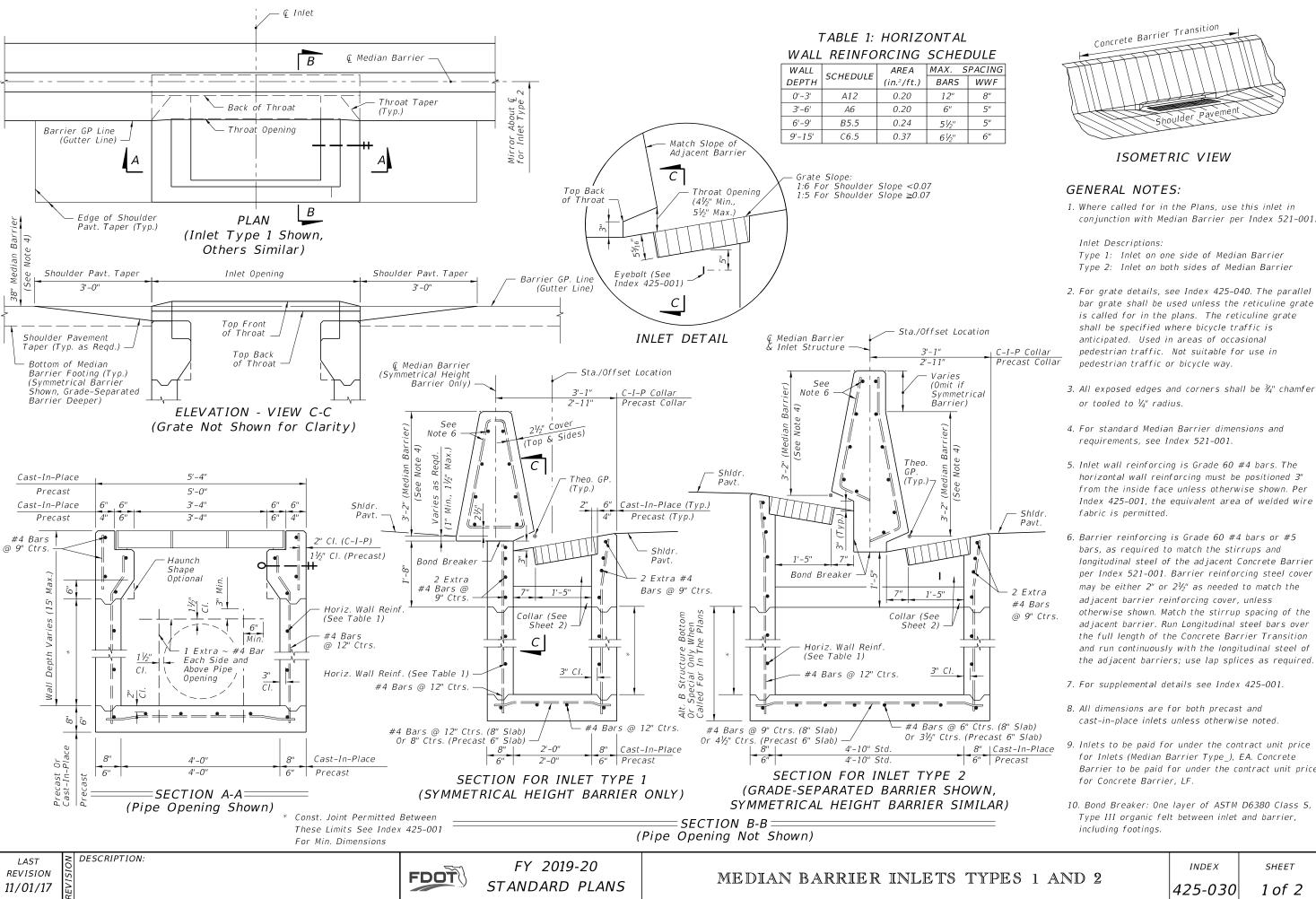


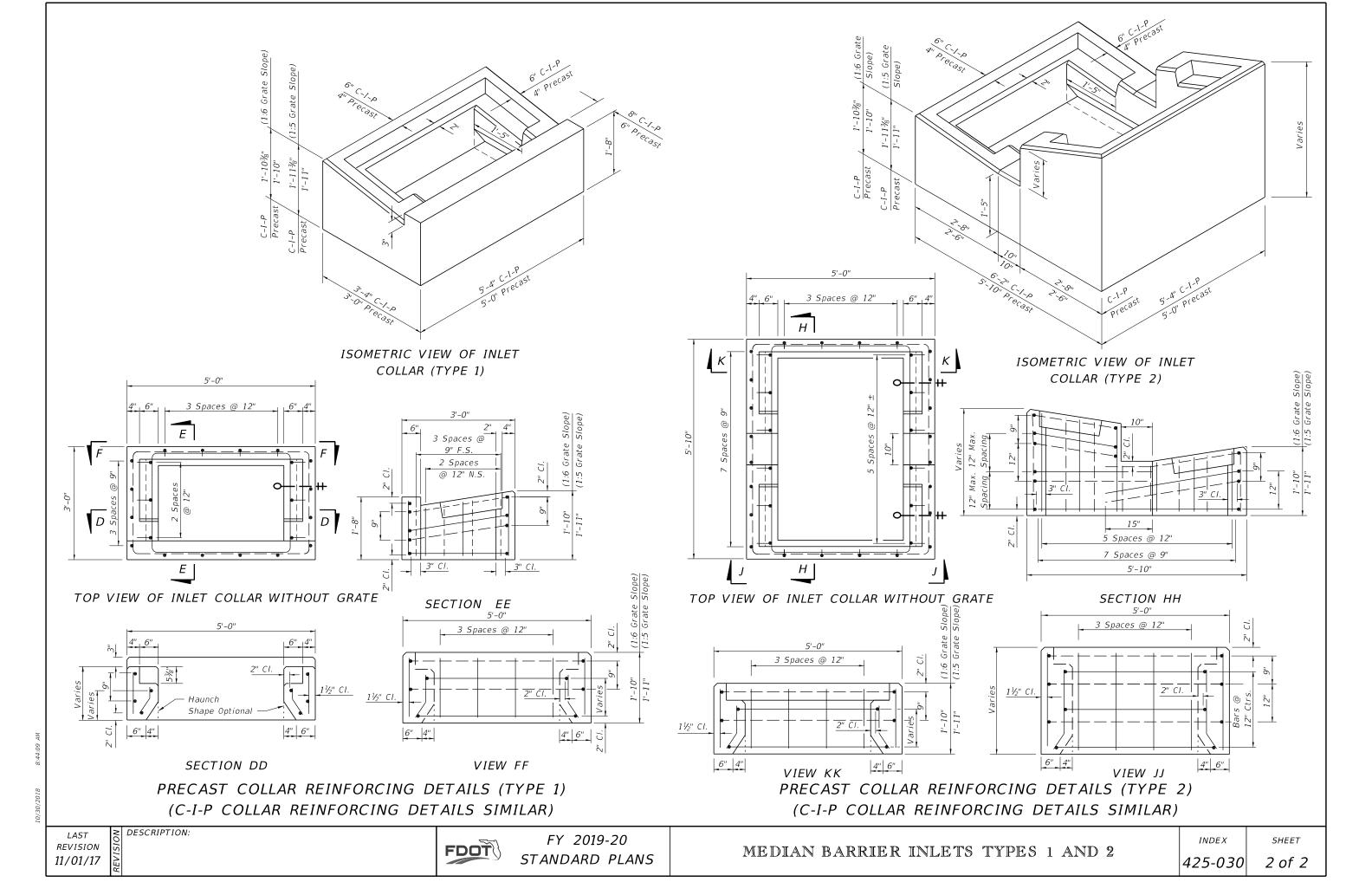




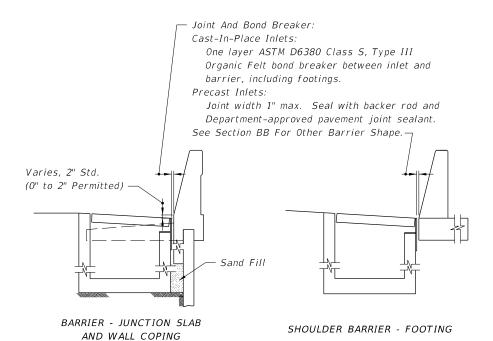


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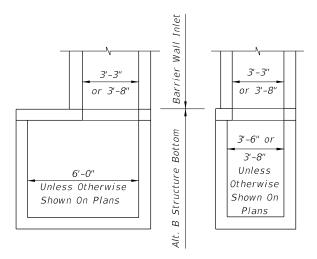




#### LOW SIDE SUPERELEVATION HIGH SIDE TRANSITION PAVEMENT WARP FOR SHOULDERS IN SUPERELEVATION



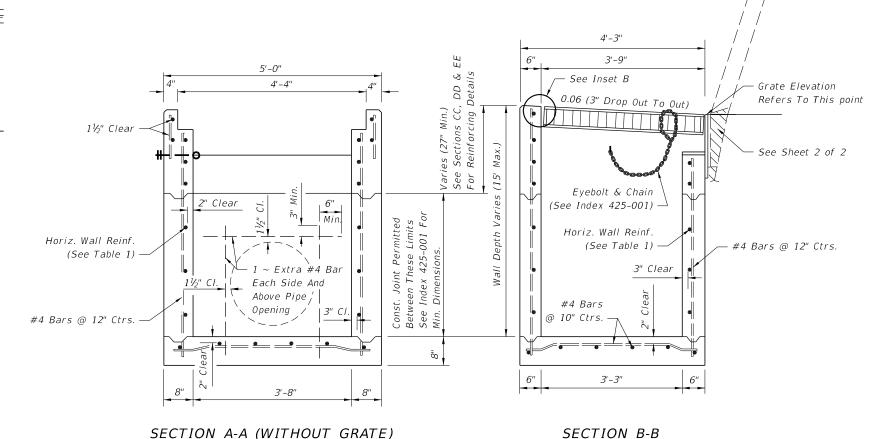
INLET SECTION AT BARRIERS



Note: Alt. B Structure Bottom Only. See Index 425-010

#### INLET WITH STRUCTURE BOTTOM

DESCRIPTION:

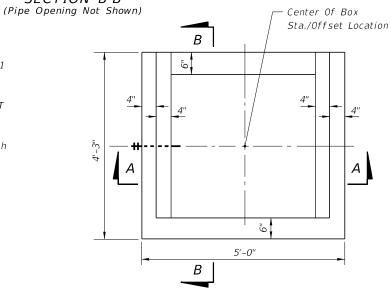


#### GENERAL NOTES:

1. Where called for in the Plans, use this inlet in conjunction with Shoulder Barrier per Index 521-001 or a Wall Coping with Barrier and Junction Slab per Index 521-610. Use of the inlet adjacent to other Concrete Barrier or Traffic Railing types requires approval of the Drainage Engineer. The inlet is suitable for bicycle and occasional pedestrian traffic, with roller bar installation (see INSET B), but should not be placed in a designated pedestrian travel way.

(Pipe Opening Shown)

- 2. Inlets located in embankments constructed with earth anchored retaining wall shall be designed with minimum depths to reduce adverse impact on the anchorage system. Runs of pipe parallel to and near anchored wall shall be avoided wherever practical. Special coordination must be exercised during the design and construction of storm water systems within anchored wall systems.
- 3. Inlet bottoms and/or tops may be either precast or cast-in-place. Whether cast as a single unit or as multiple segments, and whether precast or cast-in-place, the upper 2'-3" of the inlet shall be reinforced in accordance with sections CC, DD and EE.
- 4. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 5. When Alternate G grate is specified in the plans, the grate is to be hot-dip galvanized after fabrication. Field installation of the filler bar called for in Inset B will not be permitted, thereby requiring tolerance adjustment during fabrication and/or casting, or, matching grate to structure prior to galvanizing.
- 6. All reinforcing is Grade 60 bars. See Index 425-001 for equivalent area of welded wire fabric.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 8. For supplemental details see Indexes 425-001 and 425-010.
- 9. Inlets to be paid for under the contract unit for Inlets (Concrete Barrier), Ea.



TOP VIEW (WITHOUT GRATE)

TABLE 1: HORIZONTAL WALL REINFORCING SCHEDULE

WALL	SCHEDULE	AREA	MAX. S	
DEPTH		(in.²/ft.)	BARS	WWF
0'-5'	A12	0.20	12"	8"
5'-10'	A6	0.20	6"	5"
10'-15'	A4	0.20	4"	3"
10'-15'	B5.5	0.24	5½"	5"

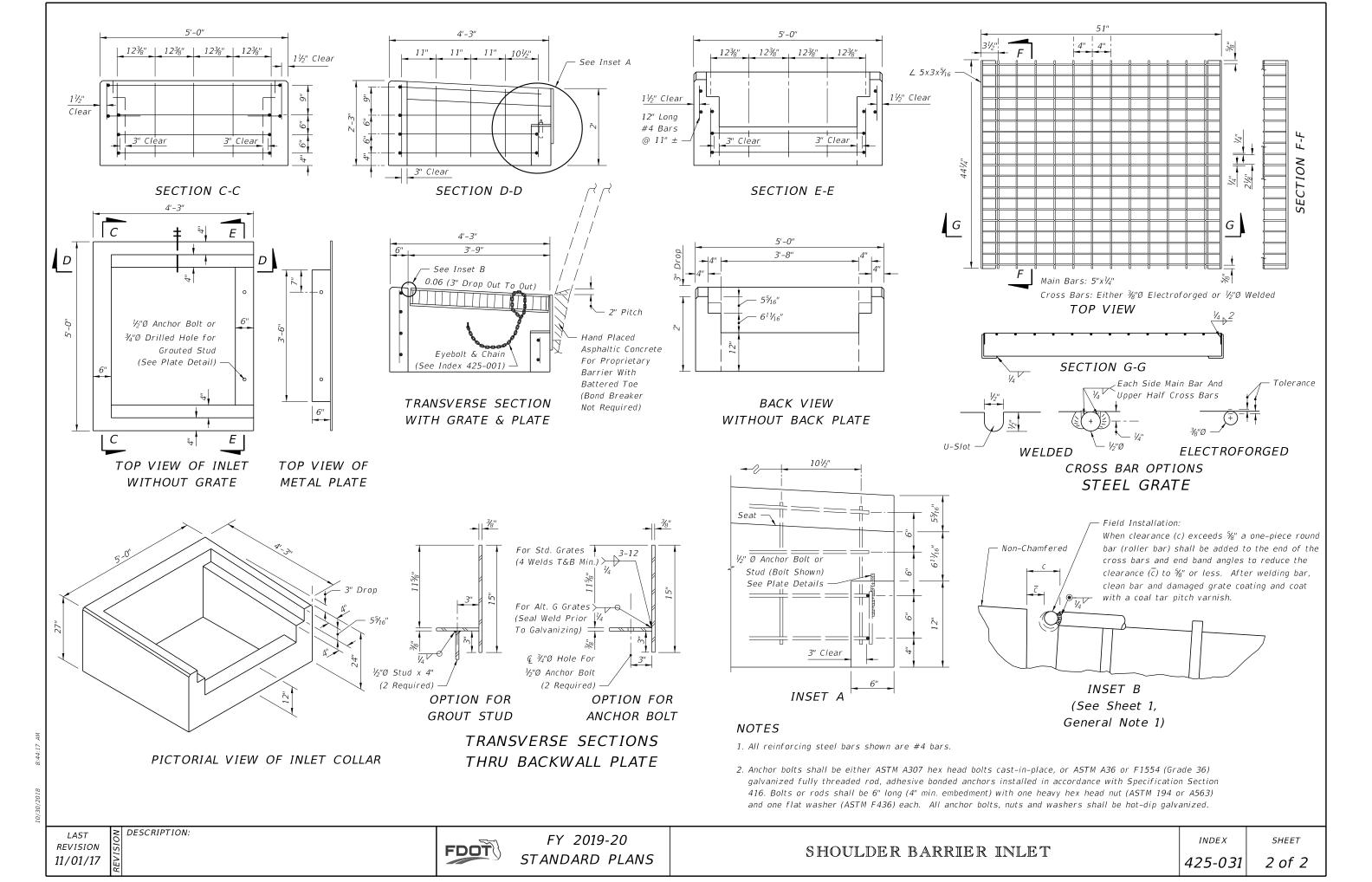
**REVISION** 12/07/17

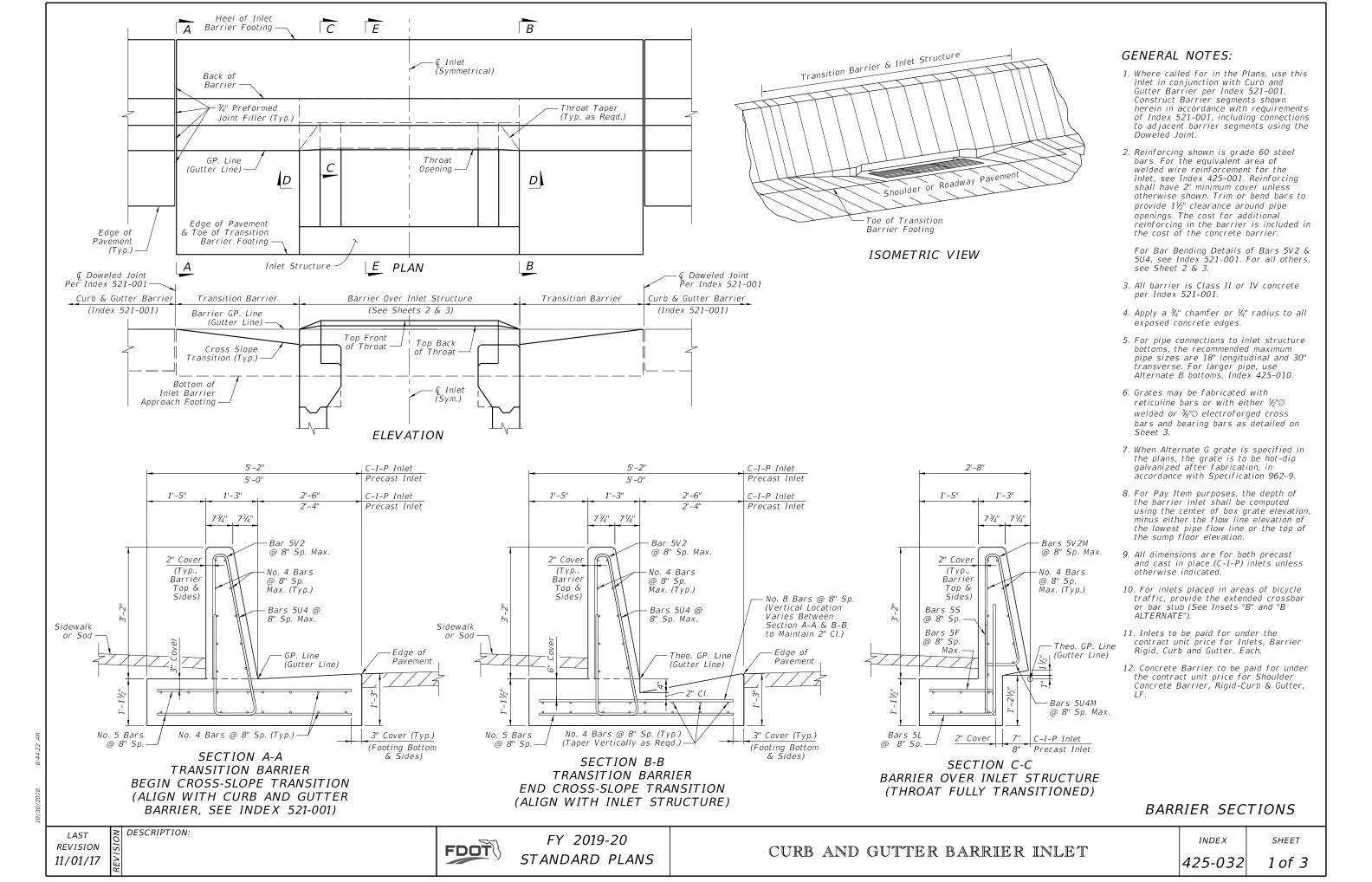
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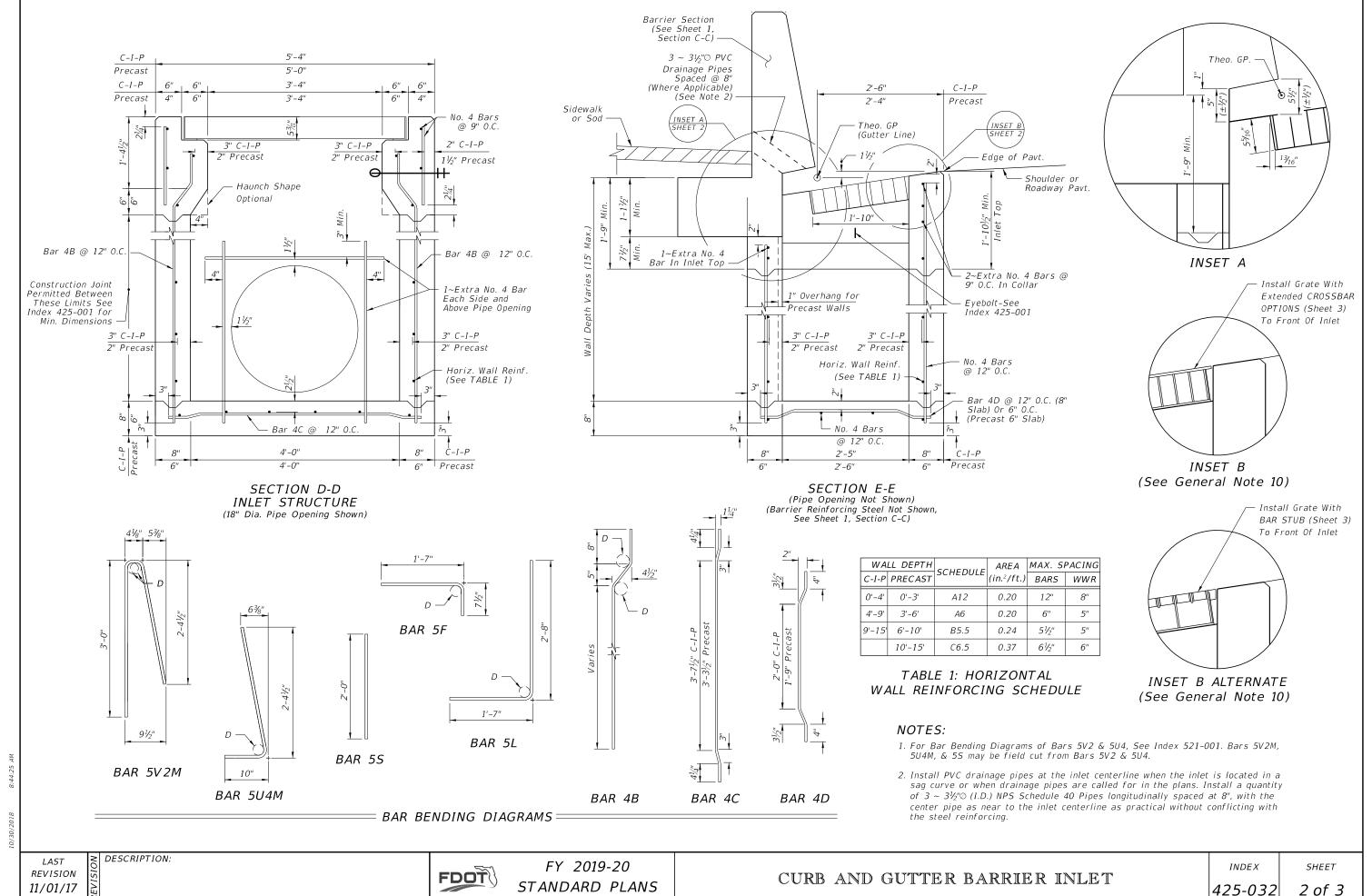
FY 2019-20 STANDARD PLANS

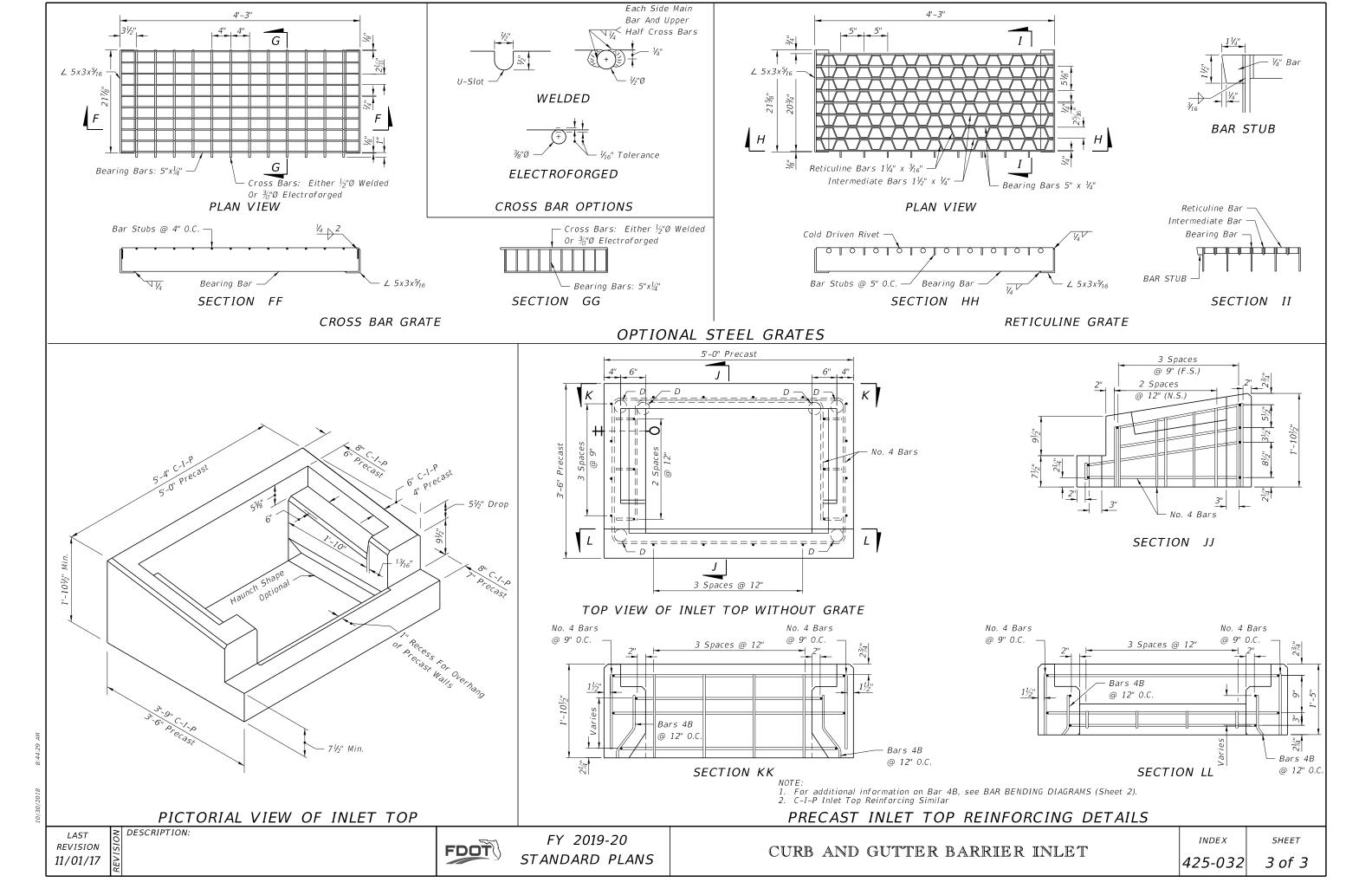
INDEX 425-031

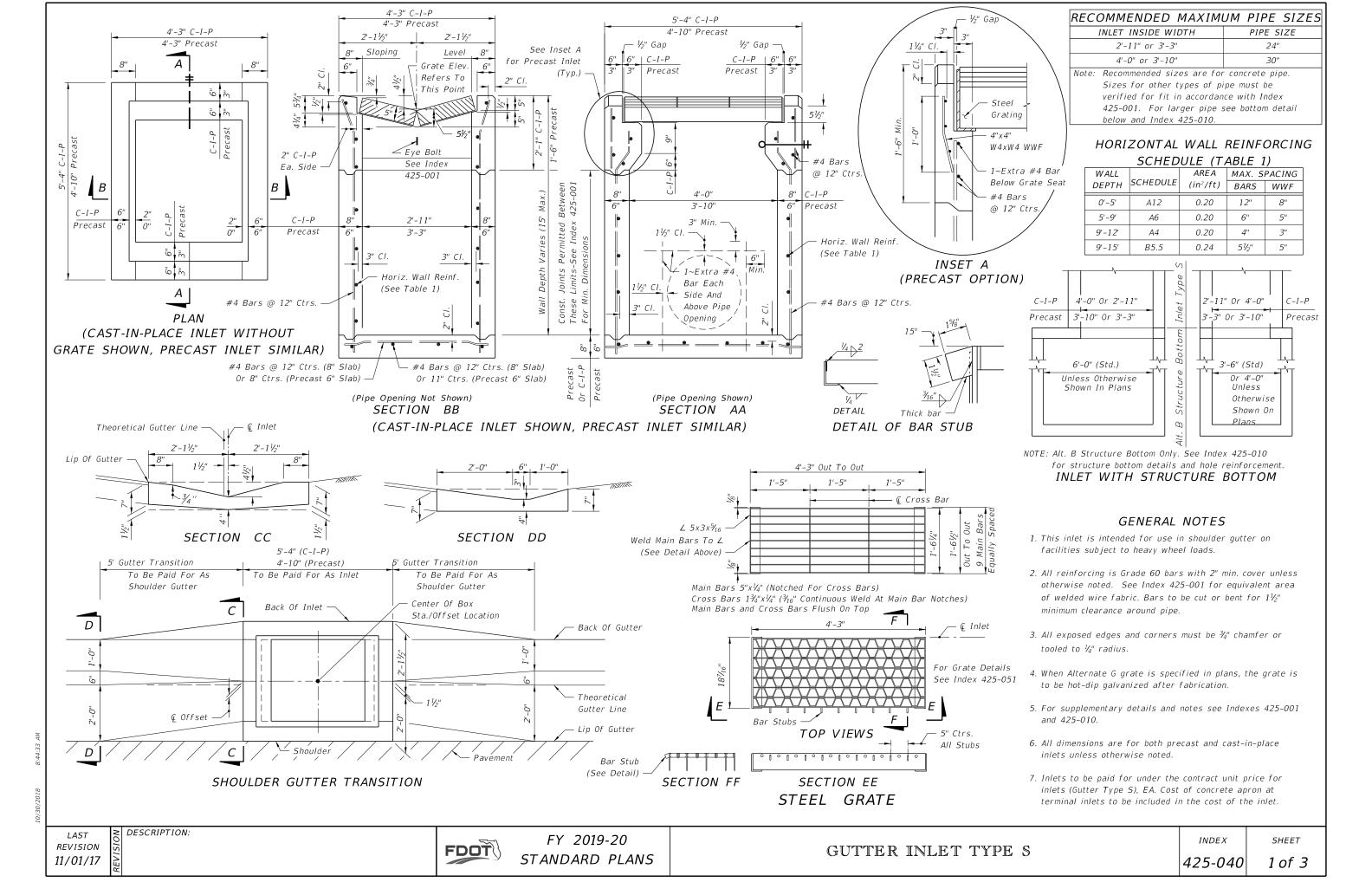
SHEET 1 of 2

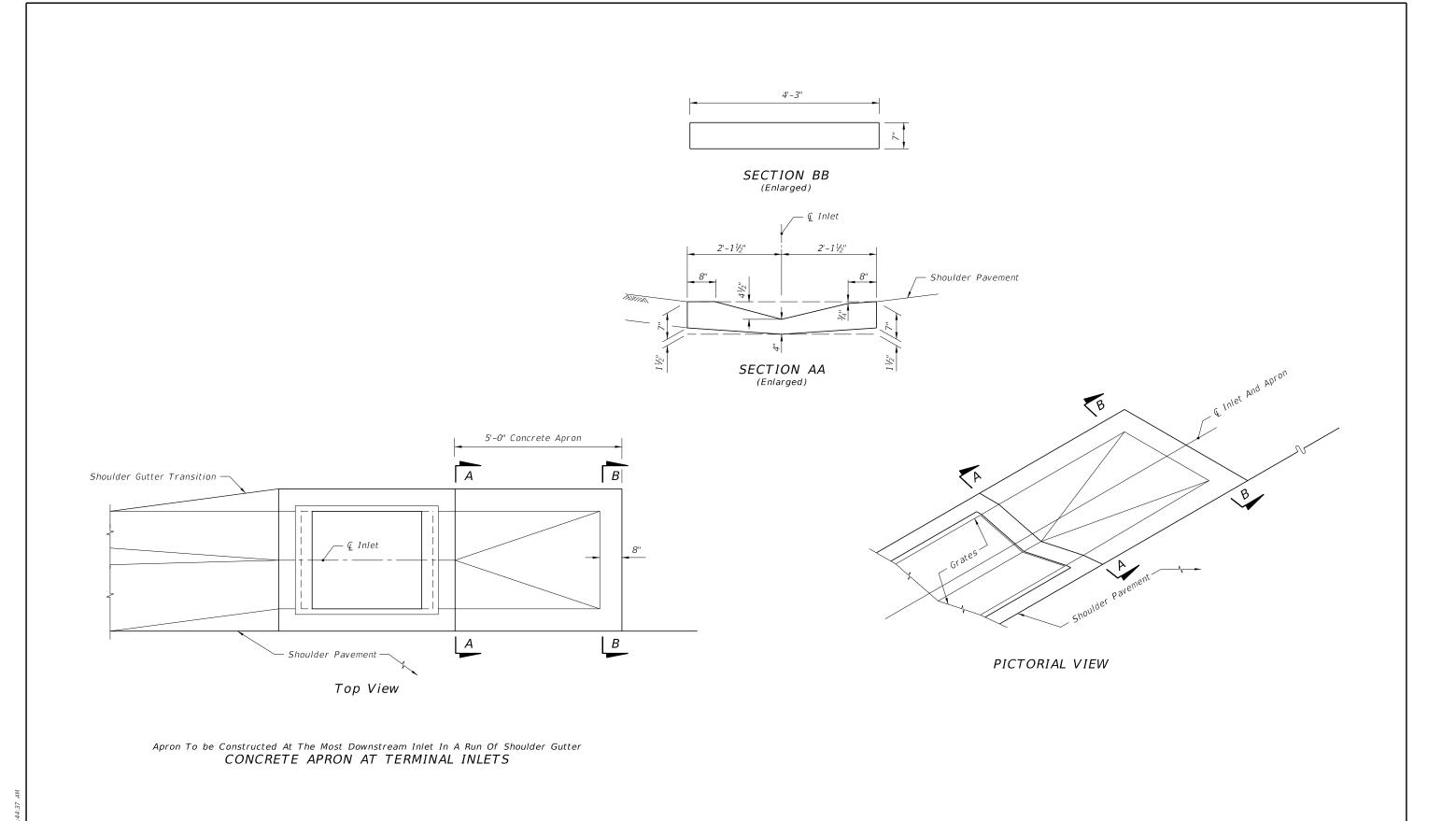












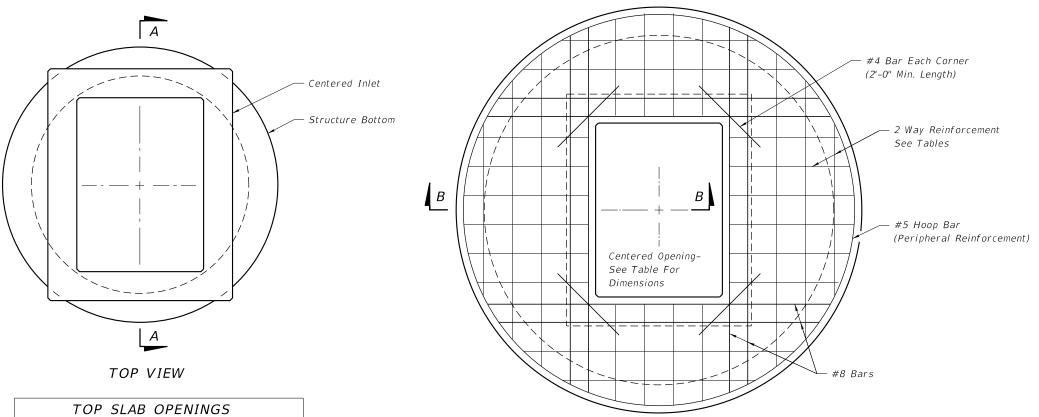
REVISION 11/01/17

≥ DESCRIPTION:

FY 2019-20 STANDARD PLANS

GUTTER INLET TYPE S

INDEX 425-040 SHEET

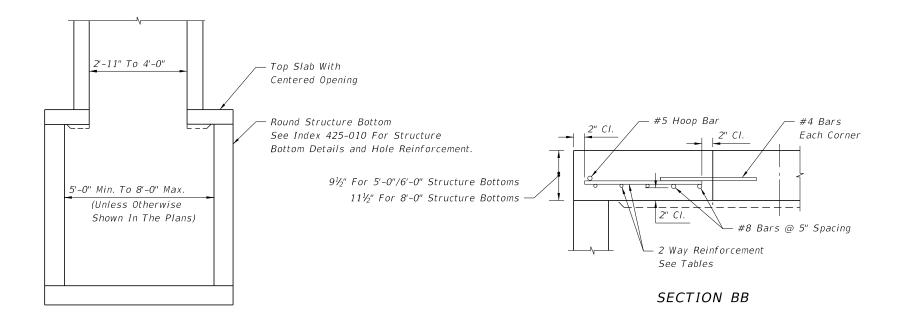


TOP SLAB		
REINFORCING SCHEDULE		
GRADE 60 (BAR)		
OR 65 KSI & 70 KSI		
(WIRE FABRIC)		
In²/ft.		
0.20		
0.24		
0.37		
0.53		
0.73		
1.06		
1.45		

TOP SLAB OPENINGS				
DIAMETER	OPENING SIZE			
	MIN. MAX.			
5'-0" To 8'-0"	2'-11" x 4'-0"	3'-3" x 3'-10"		

SECTION AA

TOP SLAB REINFORCING DIAGRAM



TOP SLAB WITH					
CEN	CENTERED OPENING				
SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE			
	SIZE: 5'-0"				
≥0.5′<30′	9½"	С			
30'-40'	9½"	D			
SIZE: 6'-0"					
≥0.5′<8′	91/2"	В			
8'<18'	9½"	С			
18'<30'	9½"	D			
30'<37'	9½"	Е			
37'-40'	9½"	G			
SIZE: 8'-0"					
≥0.5′<9′	11½"	С			
9'<15'	11½"	D			
15'<23'	11½"	E			
23'<33'	11½"	Е			
33'-40'	11½"	G			

ALT. A STRUCTURE BOTTOM FOR INLET TYPE S

REVISION 11/01/17

≥ DESCRIPTION:

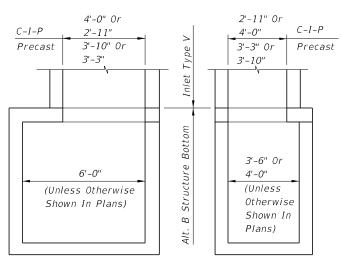
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FY 2019-20 STANDARD PLANS

GUTTER INLET TYPE S

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SHEET 3 of 3

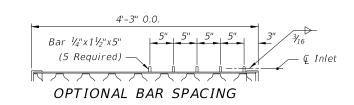


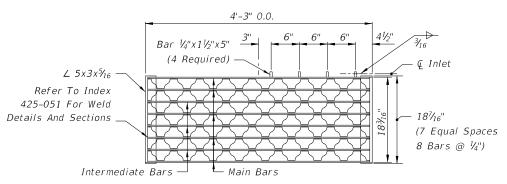
NOTE: Alt. B Structure Bottom Only. See Index 425-010 for structure bottom details and hole reinforcement.

# (For Pipes 30" Dia. And Larger) INLET WITH STRUCTURE BOTTOM

# RECOMMENDED MAXIMUM PIPE SIZES Inlet Inside Width Pipe Size 2'-11" Or 3'-3" 24" 4'-0" Or 3'-10" 30"

Note: Recommended sizes are for concrete pipe. Sizes for other types of pipe must be verified for fit in accordance with Index 425-001. For larger pipe see bottom detail above and Index 425-010.





#### TWO REQUIRED PER INLET

5" Steel Grate: Main Bars  $5"xV_4"$ Intermediate Bars  $1V_2"xV_4"$ Reticuline Bars  $1V_4"xV_{16}"$ 

Inlet Elevation As

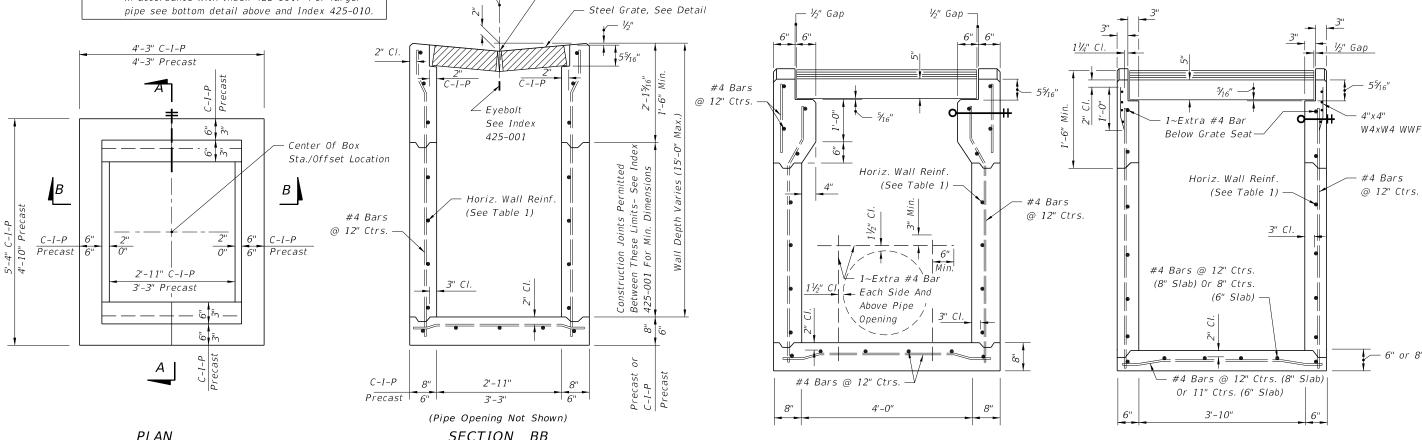
Shown On Plans

#### GENERAL NOTES

- 1. This inlet is suitable for village swales, ditches, or other areas subject to heavy wheel loads, minimum debris. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet. This inlet is not for use in a bicycle way.
- 2. When alternate "G" grate is specified in plans, the grate is to be hot dip galvanized after fabrication.
- 3. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. See Index 425-001 for equivalent area of welded wire fabric. Cut or bend bars out of way of pipe to clear pipe 1½".
- 4. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 5. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 6. For supplementary details see Index 425-001.
- 7. Inlet to be paid for under the contract unit price for Inlets (Gutter Type V), EA

# HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCIILDULL	(in.²/ft.)	BARS	WWF
0' - 5'	A12	0.20	12"	8"
5' - 9'	A6	0.20	6"	5"
9' - 12'	A4	0.20	4"	3"
9' - 15'	B5.5	0.24	5½"	5"



PLAN (CAST-IN-PLACE INLET SHOWN WITHOUT GRATE; PRECAST INLET SIMILAR)

DESCRIPTION:

SECTION BB (CAST-IN-PLACE INLET SHOWN PRECAST INLET SIMILAR)

(Pipe Opening Shown)
SECTION AA
(CAST-IN-PLACE INLET)

(Pipe Opening Not Shown)
SECTION AA
(PRECAST INLET)

LAST REVISION 11/01/17

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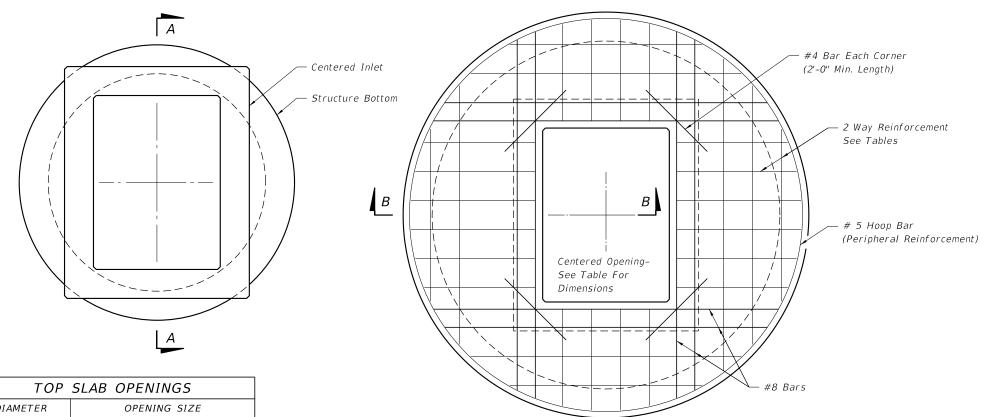
← Inlet

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GUTTER INLET TYPE V

INDEX 425-041

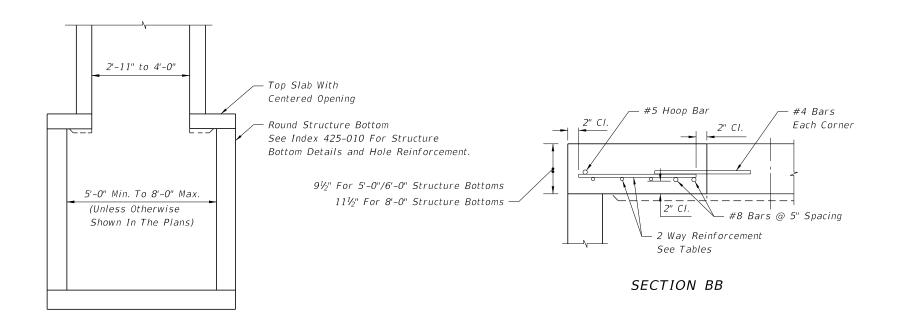
_{SHEET} 1 of 2



DIAMETER	OPENING SIZE	
	MIN.	MAX.
5'-0" To 8'-0"	2'-11" x 4'-0"	3'-3" x 3'-10"

SECTION AA

TOP SLAB REINFORCING DIAGRAM



7	TOP SLAB			
REINFOF	REINFORCING SCHEDULE			
	GRADE 60 (BAR)			
SCHEDULE	OR 65 KSI &			
SCHEDULE	70 KSI (WIRE FABRIC)			
	In.²/ft.			
Α	0.20			
В	0.24			
С	0.37			
D	0.53			
Е	0.73			
F	1.06			
G	1.45			

TOP SLAB WITH CENTERED OPENING				
SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE		
	SIZE: 5'-0'	,		
≥0.5′ <30′	91/2"	С		
30'-40'	91/2"	D		
	SIZE: 6'-0'	1		
0.5' < 8'	91/2"	В		
8' < 18'	91/2"	С		
18' < 30'	91/2"	D		
30' < 37'	91/2"	E		
37'-40'	91/2"	G		
SIZE: 8'-0"				
≥0.5′ < 9′	111/2"	С		
9' < 15'	111/2"	D		
15' < 23'	111/2"	Е		
23' < 33'	111/2"	Е		
33'-40'	111/2"	G		
	-	-		

ALT. A STRUCTURE BOTTOM FOR INLET TYPE V

REVISION 11/01/17

≥ DESCRIPTION:

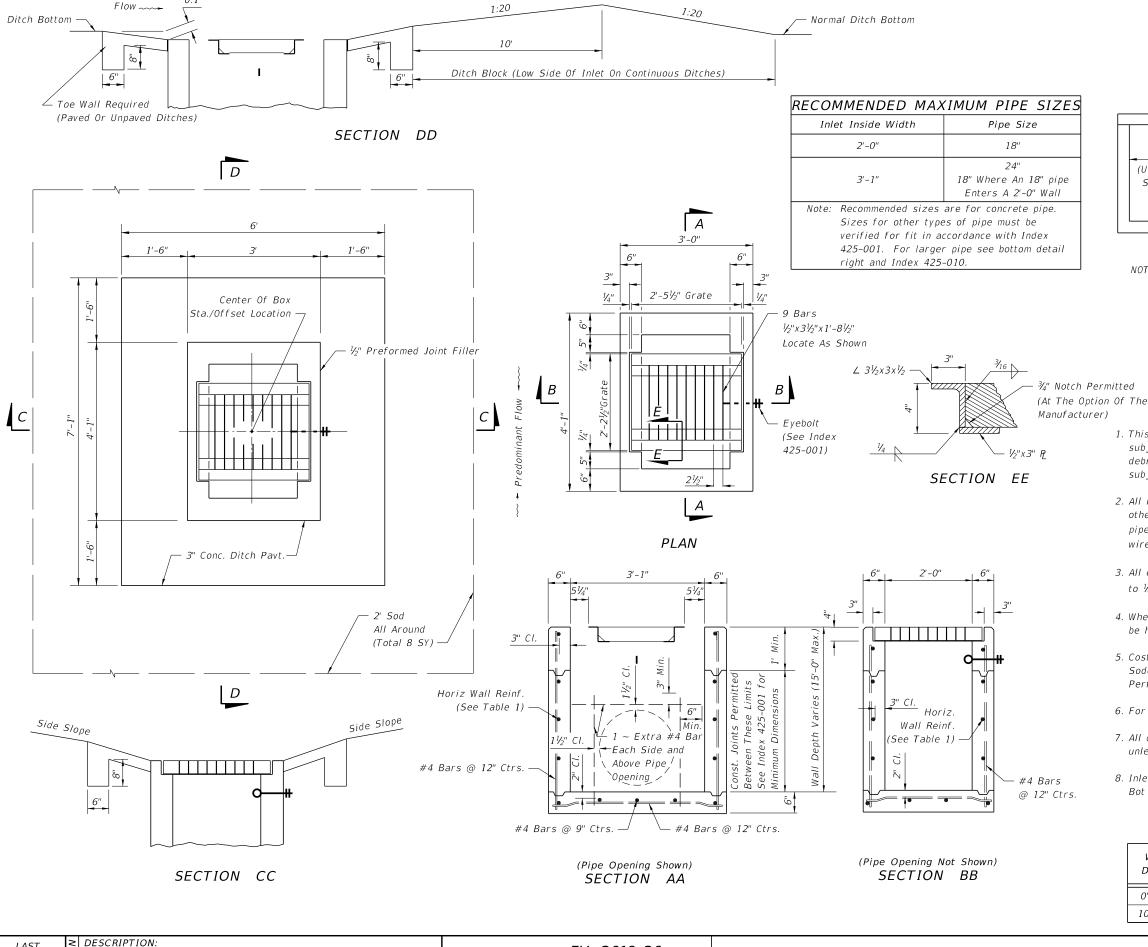
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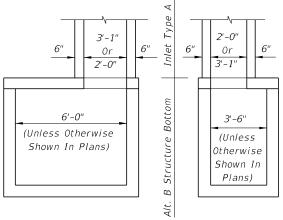
FY 2019-20 STANDARD PLANS

GUTTER INLET TYPE V

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SHEET 2 of 2





NOTE: Alt. B Structure Bottom Only. See Index 425-010 for Structure Bottom Details And Hole Reinforcement.

#### INLET WITH STRUCTURE BOTTOM

#### GENERAL NOTES

- 1. This inlet is designed for ditches, medians, or other area subject to heavy wheel loads on limited access facilities where debris may be a problem. This inlet is not for use in areas subject to pedestrian and/or bicycle traffic.
- 2. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Cut or bend bars out of way of pipe to clear pipe by 1½". See Index 425-001 for equivalent area of welded wire fabric.
- 3. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 4. When alternate "G" grate is specified in plans, the grate is to be hot-dip galvanized after fabrication.
- Cost of ditch paving to be included in the cost of Inlet.
   Sodding to be paid for under contract unit price for Performance Turf, SY.
- 6. For supplemental details see Index 425-001.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 8. Inlet to be paid for under the contract unit price for inlets (Dt Bot Type A), EA.

# HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0' - 10'	A12	0.20	12"	8"
10' - 15'	A6	0.20	6"	5"

LAST REVISION 11/01/17

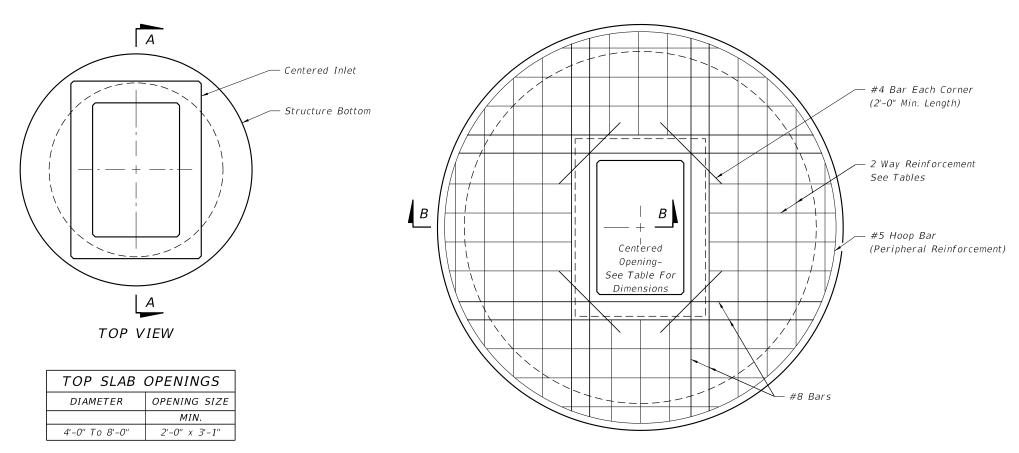
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FY 2019-20 STANDARD PLANS

DITCH BOTTOM INLET TYPE A

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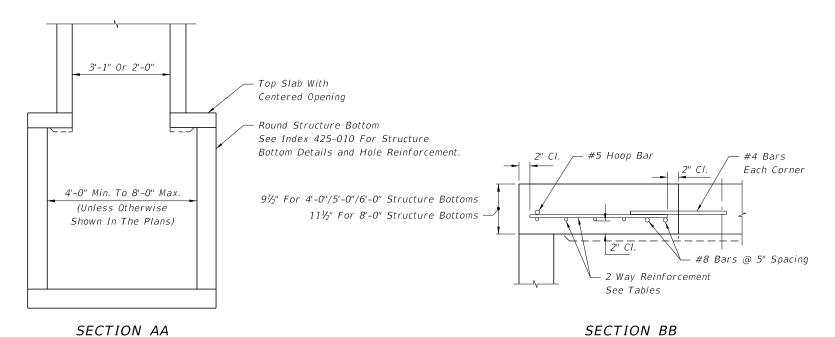
*SHEET*1 of 2



REINFORCING SCHEDULE           GRADE 60 (BAR) OR           65 KSI & 70 KSI           (WIRE FABRIC)           In.²/ft.           A         0.20           B         0.24           C         0.37           D         0.53           E         0.73           F         1.06	T	TOP SLAB		
SCHEDULE 65 KSI & 70 KSI (WIRE FABRIC) In.²/ft.  A 0.20 B 0.24 C 0.37 D 0.53 E 0.73	REINFORG	REINFORCING SCHEDULE		
A 0.20 B 0.24 C 0.37 D 0.53 E 0.73	SCHEDULE	65 KSI & 70 KSI (WIRE FABRIC)		
B 0.24 C 0.37 D 0.53 E 0.73		<u> </u>		
C 0.37 D 0.53 E 0.73	A	0,=0		
D 0.53 E 0.73	В	0.24		
E 0.73	С	0.37		
	D	0.53		
F 1.06	Ε	0.73		
	F	1.06		
G 1.45	G	1.45		

TOP SLAB WITH					
CEI	NTERED OP	ENING			
SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE			
	SIZE: 4'-0"				
≥0.5′-40′	9½"	С			
	SIZE: 5'-0"				
≥0.5′<30′	9½"	С			
30'-40'	9½"	D			
SIZE: 6'-0"					
0.5'<8'	9½"	В			
8'<18'	9½"	С			
18'<30'	9½"	D			
30'<37'	9½"	E			
37'-40'	9½"	G			
SIZE: 8'-0"					
≥0.5′<9′	11½"	С			
9'<15'	11½"	D			
15'<23'	11½"	Е			
23'<33'	11½"	Е			
33'-40'	11½"	G			

TOP SLAB REINFORCING DIAGRAM



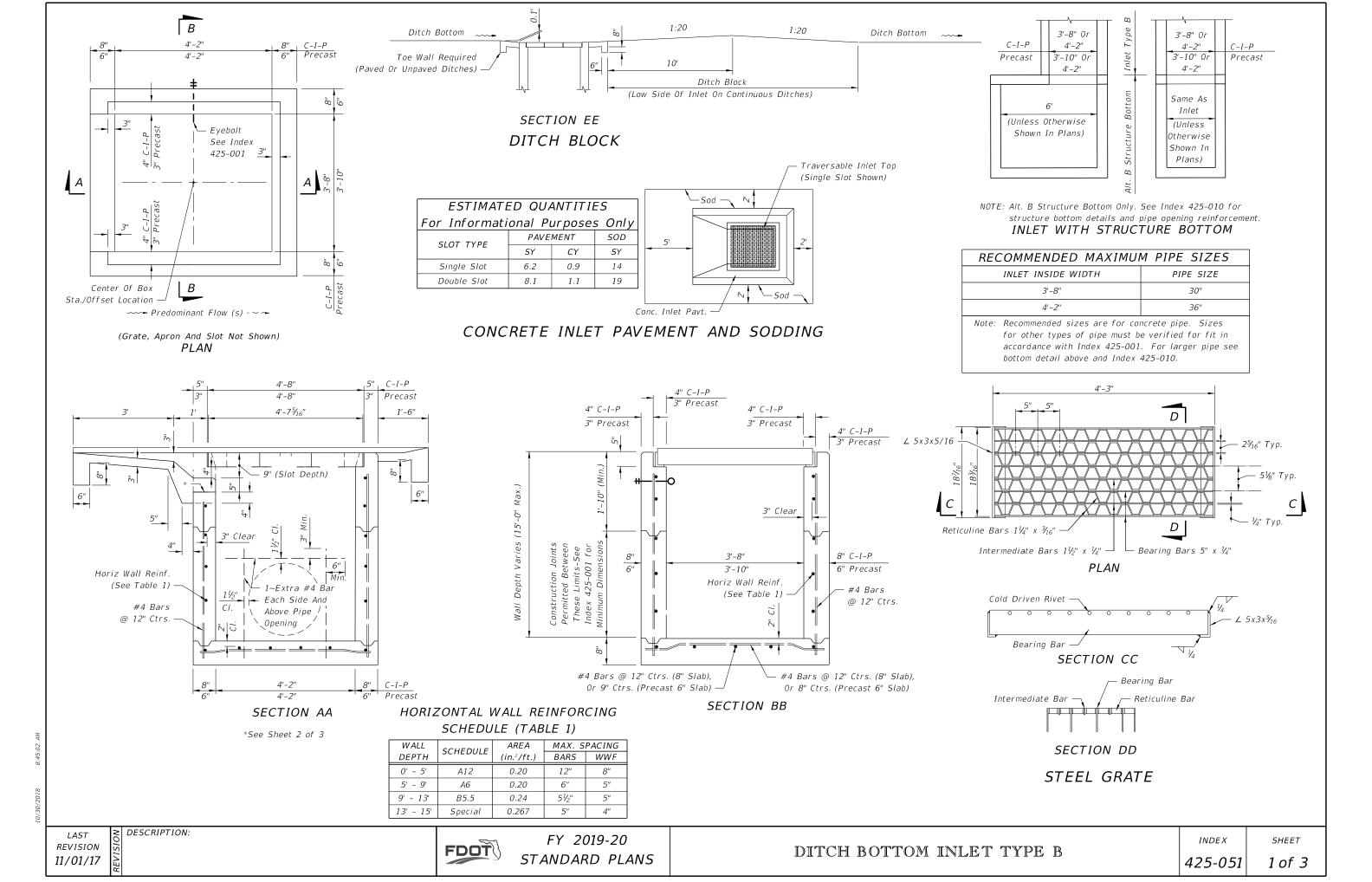
ALT. A STRUCTURE BOTTOM FOR INLET TYPE A

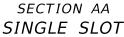
REVISION 11/01/17

≥ DESCRIPTION:

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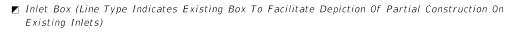
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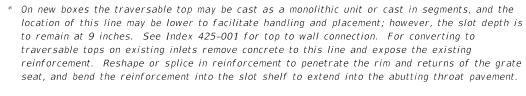


4" C-I-P or

SECTION BB



DOUBLE SLOT



TRAVERSABLE TOPS FOR INLETS TYPE B AND FOR CONVERSIONS OF EXISTING INLETS TYPE B AND TYPE X

#### GENERAL NOTES

- 1. The general purpose of the inlet top designs are:
  - a. For ditches, medians or other areas subject to heavy wheel loads. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet. Inlet not suitable for bicycle traffic.
  - b. Provide full grate and horizontal slot designs for new construction.
  - c. Provide full grate and horizontal slot designs for replacing the vertical slot tops on existing Inlets Type B and Type X that are in locations subject to occasional pedestrian traffic.
- 2. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. See Index 425–001 for equivalent area of welded wire fabric. Bars to be cut or bent for min.  $1\frac{1}{2}$ " clearance around pipe.
- 3. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 4. When Alternate G grates are specified in the plans, the grates are to be hot-dip galvanized after fabrication.
- 5. Cost for constructing traversable tops on new inlet boxes shall be included in the contract unit price for Inlets (DT BOT) (Type B), EA., and shall include the cost for surrounding concrete inlet pavement. Existing Inlets Type B and Inlets Type X that are converted to traversable inlet tops shall be paid for under the contract unit price for Inlets (DT BOT) (Type B) (Partial), EA. Unit price and payment shall be full compensation for inlet conversion and shall include the removal and disposal of any existing concrete inlet pavement; the removal and stockpiling or disposal of sufficient material from the existing inlet box to facilitate construction of the required inlet top; construction of the required inlet conversion; backfill construction; construction of concrete inlet pavement; reusing, supplementing, transferring or replacing grates as required by plans or as directed by the Engineer; any required earthwork for ditch restoration within 30' of the inlet; and, restoration of disturbed turf.
- 6. Ditch pavement shall be paid for, separate from the inlet and concrete inlet pavement, by pavement types and units as called for in the plans.
- 7. Sod will be paid for under the contract unit price for Performance Turf, SY.
- 8. For supplementary details see Index 425-001.
- 9. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.

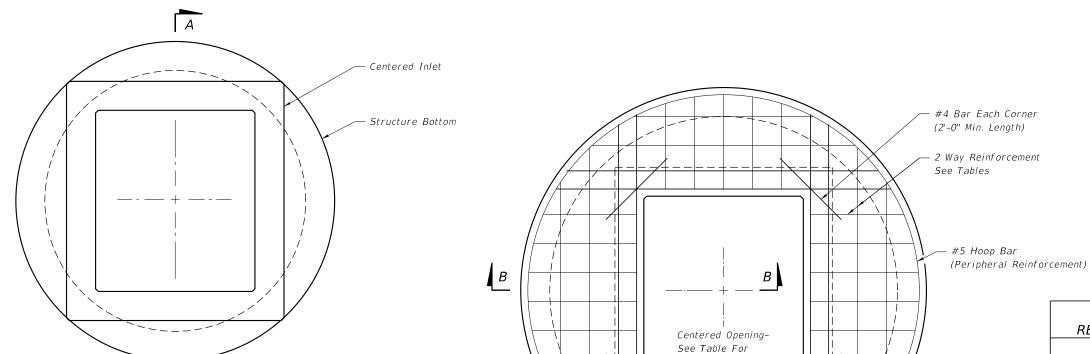
#### DESIGN NOTES

- 1. The type of top (single or double slots) depends on the approach ditch configuration and the hydraulic requirements of the site. The designer will stipulate in the plans the type of top to be constructed at each individual inlet location.
- 2. On existing inlets, conversion grates shall be constructed at the original grate elevations unless other elevations are called for in the plans. When plans call for the inlet top to be constructed to support storm water detention, details for ditch modifications and underdrains shall be shown in the plans.

#### MAINTENANCE NOTES

1. Traversable inlet tops that are constructed by maintenance contract or by maintenance forces may reuse the existing grates that are determined by the Maintenance Engineer to be functionally sound, and their reuse is so directed by the Maintenance Engineer. Existing grates approved for reuse and new grates may be mixed, matched or replaced as directed by the Maintenance Engineer.

8:45:07 AM



Dimensions

TOP SLAB REINFORCING DIAGRAM

SECTION BB

TOP SLAB OPENINGS				
DIAMETER OPENING SIZE				
MIN. MAX.		MAX.		
6'-0" to 8'-0"	3'-8" x 4'-2"	3'-10" x 4'-2"		

TOP VIEW

SECTION AA

TOP SLAB REINFORCING SCHEDULE

SCHEDULE	GRADE 60 (BAR) OR 65 KSI & 70 KSI (WIRE FABRIC)
	In²/ft.
А	0.20
В	0.24
С	0.37
D	0.53
Ε	0.73
F	1.06
G	1.45

C-I-P Precast	3'-8" Or 4'-2" 3'-10" Or 4'-2"  6'-0" Min. To 8'-0" Max. (Unless Otherwise Shown In The Plans)	Top Slab With Centered Opening  Round Structure Bottom See Index 425-010 For Structure Bottom Details and Hole Reinforcement.  9½" For 6'-0" Structure Bottoms 11½" For 8'-0" Structure Bottoms 2" Cl. #5 Hoop Bar 2" Cl. #8 Bars @ 5" Spacing 2 Way Reinforcement See Tables

TOP SLAB WITH					
CEN	CENTERED OPENING				
SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE			
	SIZE: 6'-0"				
0.5' < 8'	91/2"	В			
8' < 18"	9½"	С			
18' < 30'	9½"	D			
30' < 37'	9½"	E			
37'-40'	9½"	G			
	SIZE: 8'-0"				
≥0.5' < 9'	111/2"	С			
9' < 15'	11½"	D			
15' < 23'	11½"	E			
23' < 33'	11½"	E			
33'-40'	111/2"	G			

ALT. A STRUCTURE BOTTOM FOR INLET TYPE B

REVISION 11/01/17

≥ DESCRIPTION:

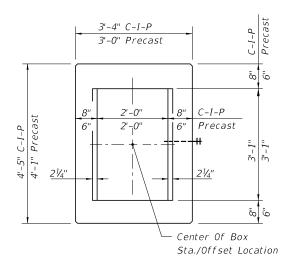
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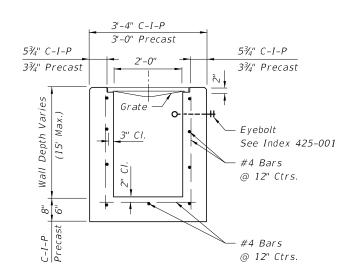
DITCH BOTTOM INLET TYPE B

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#### PLAN



**SECTION** 

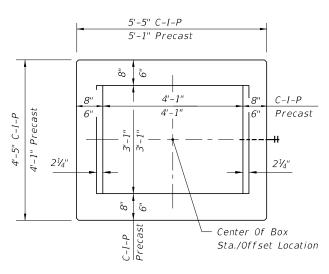
## HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 1)

WALL	SCHEDULE	AREA (in.²/ft.)	MAX. S	PACING
DEPTH	SCHEDULE		BARS	WWF
0'-15'	A12	0.20	12"	8"

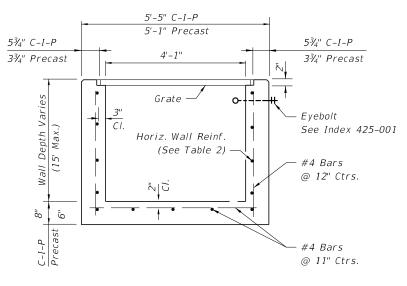
#### TYPEC

Recommended Maximum Pipe Size:

2'-0" Wall - 18" Pipe 3'-1" Wall - 24" Pipe (18" where an 18" pipe enters a 2'-0" wall)



#### PLAN



**SECTION** 

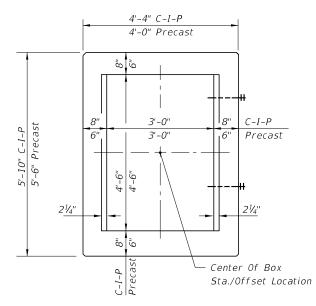
## HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 2)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH		(in.²/ft.)	BARS	WWF
0'-6'	A12	0.20	12"	8"
6'-10'	A6	0.20	6"	5"
10'-13'	A4	0.20	4"	3"
10'-15'	B5.5	0.24	5½"	5"

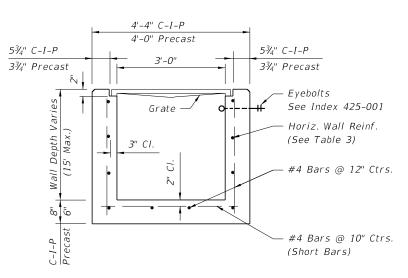
#### TYPED

Recommended Maximum Pipe Size:

3'-1" Wall - 24" Pipe 4'-1" Wall - 36" Pipe



PLAN



SECTION

## HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 3)

WALL	VALL SCHEDULE AREA		COUEDINE	MAX. S	PACING
DEPTH	SCHEDOLL	(in.²/ft.)	BARS	WWF	
0'-5'	A12	0.20	12"	8"	
0'-7.5'	A6	0.20	6"	5"	
7.5'-10'	B5.5	0.24	5½"	5"	
10'-15'	C6.5	0.37	6½"	6"	

# TYPE E

Recommended Maximum Pipe Size:

3'-0" Wall - 24" Pipe 4'-6" Wall - 36" Pipe

**REVISION** 11/01/17

DESCRIPTION:

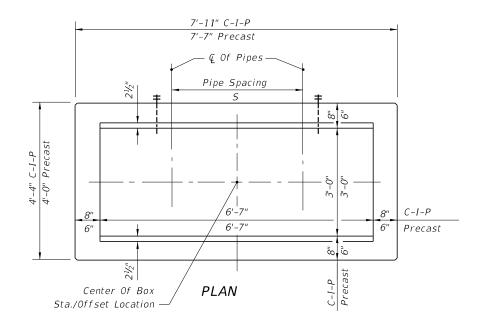
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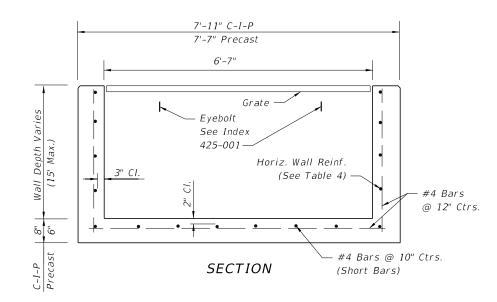
FY 2019-20 STANDARD PLANS

DITCH BOTTOM INLET TYPES C, D, E AND H

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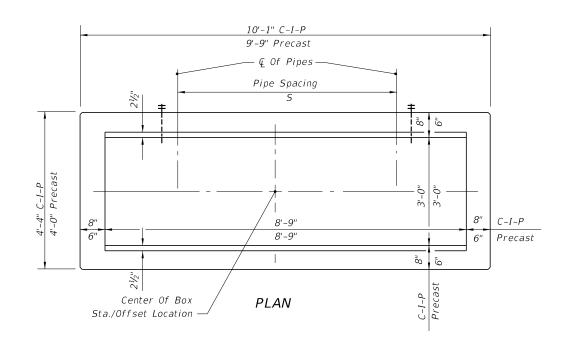


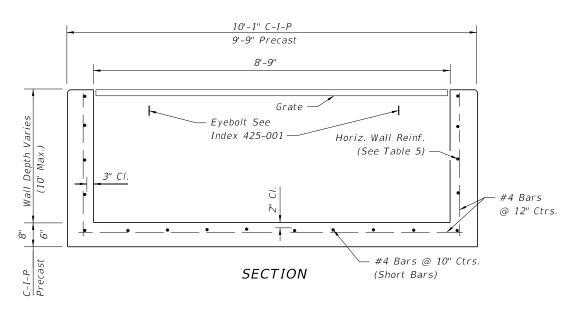
# HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 4)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0'-5'	B5.5	0.24	5½"	5"
5'-7'	C6.5	0.37	61/2"	6"
7'-15'	D4.5	0.53	41/2"	4"

# TYPE H (2 & 3-GRATE INLET)

Recommended Maximum Pipe Size: 3'-0" Wall - 24" Pipe 6'-7" Wall - 1-60" Pipe Or 2-24" Pipe (S=3'-5")





# HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 5)

WALL		AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0'-5'	C3.5	0.37	3½"	3"
5'-10'	D4.5	0.53	4½"	4"

# TYPE H (4-GRATE INLET)

Recommended Maximum Pipe Size: 3'-0" Wall - 24" Pipe 8'-9" Wall - 1-78" Pipe Or 2-30" Pipe (S=4'-3") GENERAL NOTES See Sheet 3 of 7.

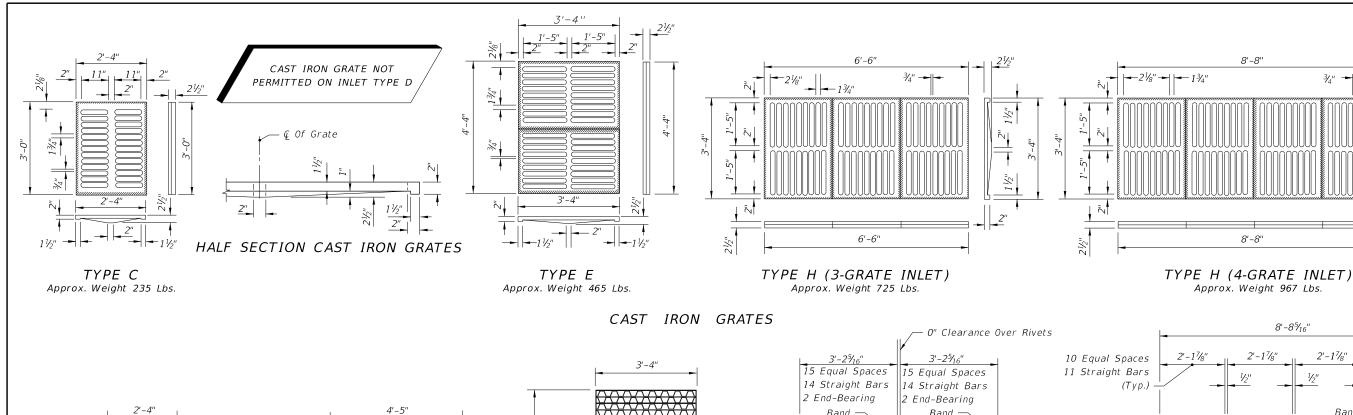
REVISION 11/01/17

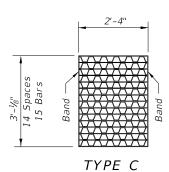
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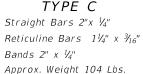
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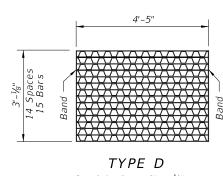
FY 2019-20 STANDARD PLANS

SHEET

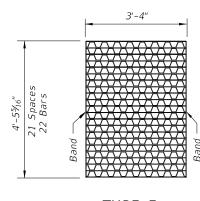




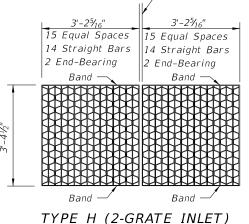




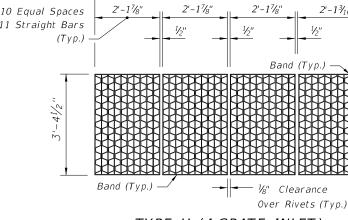
Straight Bars 2" x 1/4" Reticuline Bars 11/4" x 3/16" Bands 2" x 1/4" Approx. Weight 190 Lbs.



TYPE EStraight Bars 2" x 1/4" Reticuline Bars 11/4" x 3/16" Bands 2" x 1/4" Approx. Weight 215 Lbs.



Straight End-Bearing Bars 2" x 3/8" Straight Bearing Bars 2" x 1/4" Reticuline Bars 11/4" x 3/16"



TYPE H (4-GRATE INLET) Straight End-Bearing Bars 2" x 1/4" Reticuline Bars 11/4" x 3/16" Banding Bars 2" x 3/16" Approx. Total Weight 388 Lbs.

#### STEEL GRATES

NOTE: Steel Grates Are Required On Inlets With Traversable Slots And On Inlets where Bicycle Traffic Is Anticipated. GENERAL NOTES

- 1. These inlets are suitable for bicycle traffic and are to be used in ditches. medians and other areas subject to infrequent traffic loadings but are not to be placed in areas subject to any heavy wheel loads. These inlets may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and
- 2. Inlets subject to minimal debris should be constructed without slots. Where debris is a problem inlets should be constructed with slots. Slotted inlets located within roadway clear zones and areas subject to pedestrians shall have traversable slots. The traversable slot modification is not adaptable to inlet Type H. Slots may be constructed at either or both ends as shown on plans. Traversable slots shall not be used in areas subject to occasional bicycle

pavement areas where pedestrians can walk around the inlet.

3. Steel grates are to be used on all inlets where bicycle traffic is anticipated. Steel grates are to be used on all inlets with traversable slots. Either cast iron or steel grates may be used on inlets without slots where bicycle traffic is not anticipated. Either cast iron or steel grates may be used on all inlets with

- non-traversable slots. Subject to the selection described above, when Alternate G grate is specified in the plans, either the steel grate, hot dip galvanized after fabrication, or the cast iron grate may be used, unless the plans stipulate the
- 4. Recommended maximum pipe sizes shown are for concrete pipe. Size for other types of pipe must be checked for fit.
- 5. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 6. Concrete inlet pavement to be used on inlets without slots and inlets with non-traversable slots only when called for in the plans; but required on all traversable slot inlets. Cost to be included in contract unit price for inlets. Quantities shown are for information only.
- 7. Traversable slots constructed in existing inlets shall be paid for as inlets partial. For conversion work and method of payment see 'TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS'.

- 8. Sodding to be used on all inlets not located in paved areas and paid for under contract unit price for Performance Turf, SY.
- 9. For supplementary details see Index 425-001.
- 10. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Bars to be cut or bent for  $1\frac{1}{2}$ " clearance around pipe opening. Provide one additional #4 bar above and at each side of pipe opening.

**REVISION** 11/01/17



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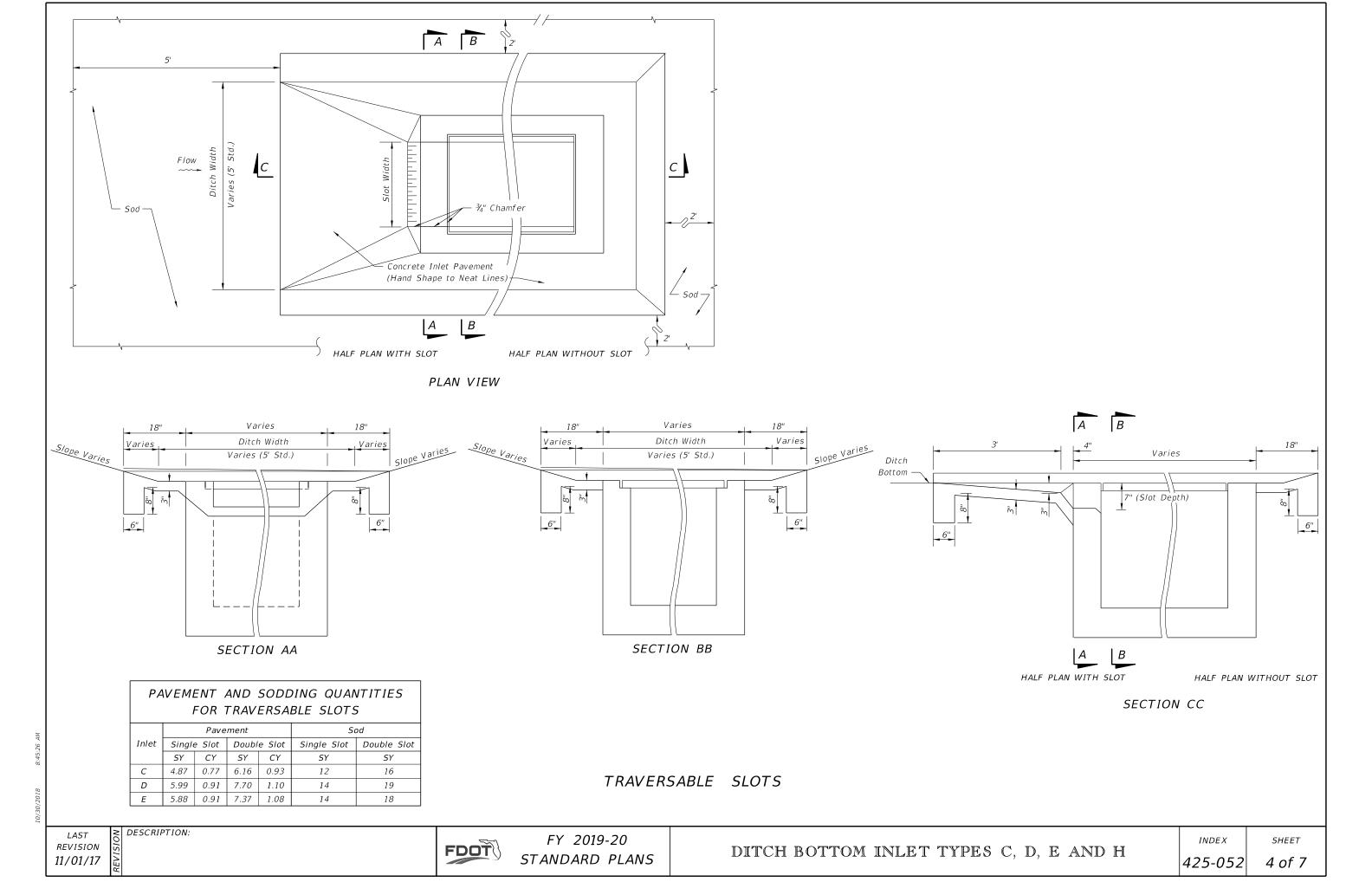
2'-13/16"

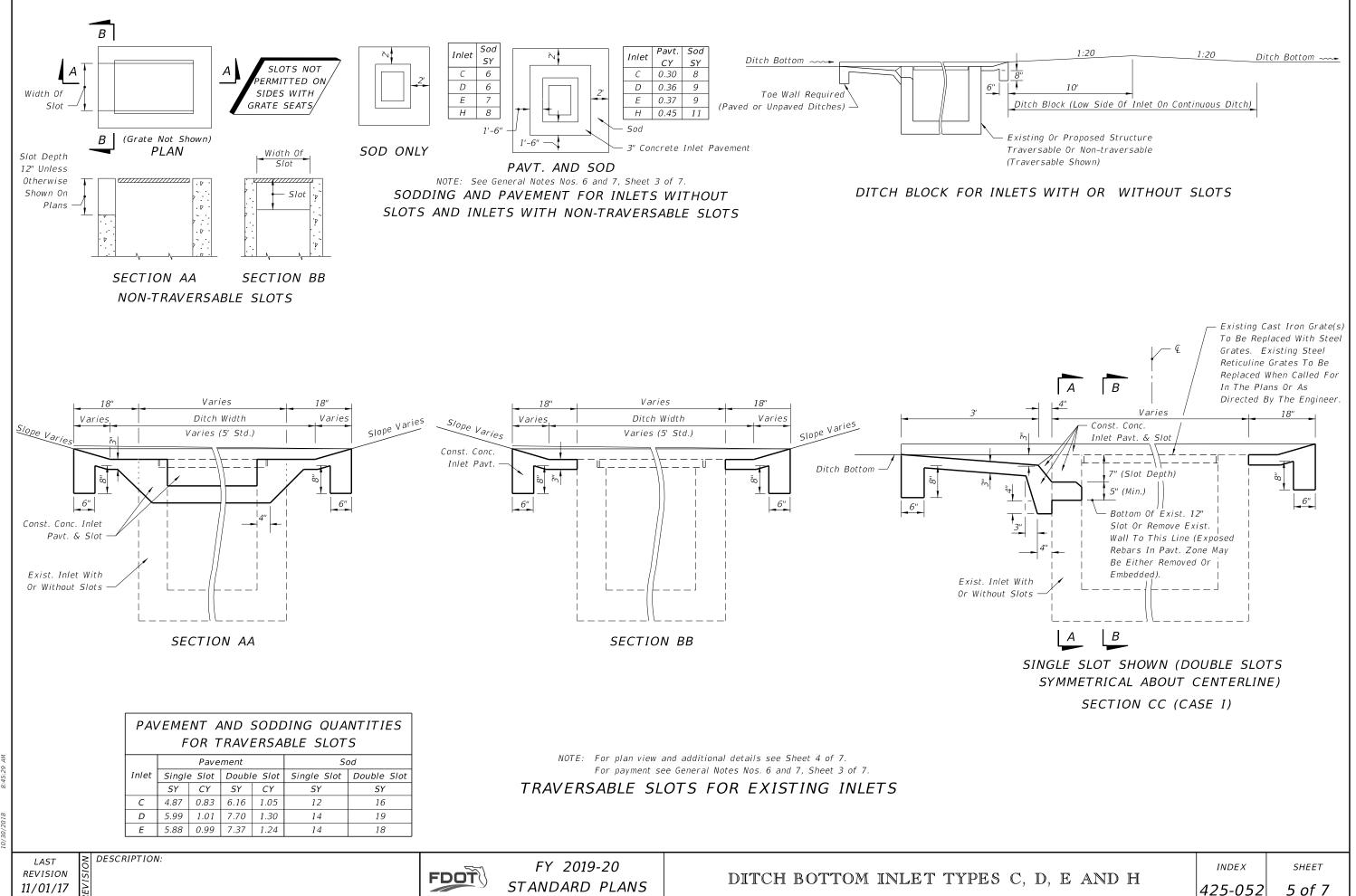
DESCRIPTION:

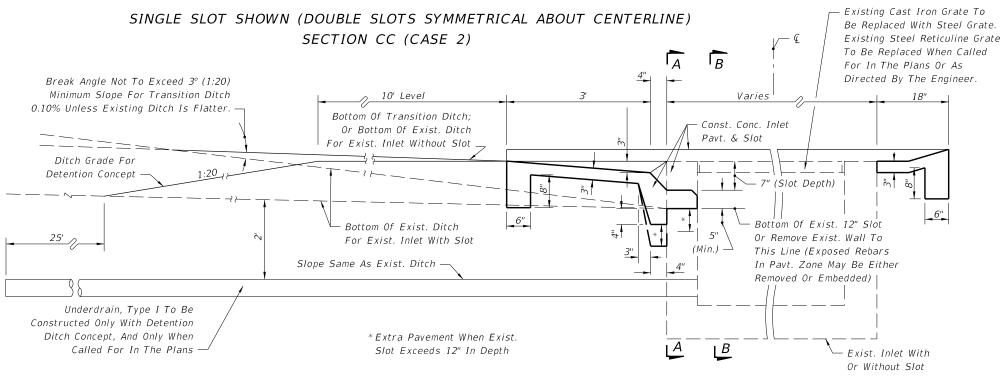
traffic.

Banding Bars 2" x 1/4"

Approx. Total Weight 310 Lbs.







SINGLE SLOT SHOWN (DOUBLE SLOTS SYMMETRICAL ABOUT CENTERLINE) SECTION CC (CASE 3)

TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

## DESIGN NOTES FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- 1. The general purpose of these conversions is to remove the hazard of the protruding inlet top, while not creating a hazard by depressing the top too deeply.
- 2. The corrective procedure depends on the approach ditch grade and hydraulic requirements of the site. The selection of the appropriate case depends on the relationship between inlet top and ditch elevation, and, on the vertical clearance between the top of the uppermost pipe(s) and the grate. The purpose for the Case 1 conversion is to add the traversable slot to an existing inlet where top removal, change in grate elevation and ditch transitions are not required. Case 2 will normally be applicable to ditches with flatter grades adjoining the inlet. Case 3 will normally be applicable to ditches with steeper grades adjoining the inlet where build up of the existing ditch is acceptable.
- 3. The designer shall stipulate in the plans which case is to be constructed at each individual inlet location.

Where the existing inlet top is above the existing ditch (Case 2) but borrow material will be required to adjust the ditch (Case 3), and vertical clearance or other conditions do not prevent removal of the inlet top, the designer should call for Case 2. The designer shall determine if ditch reconstruction is required more than 35 feet beyond any traversable slot side and shall include separate pay items in the plans to cover the cost for that portion of required ditch reconstruction exceeding the 35 foot limit. The designer shall also determine whether ditch pavement is required for ditch restoration within the 35 foot limit and include that pavement under a pay item separate from the inlets partial.

When the detention ditch concept is to be used with Case 3, the designer shall stipulate 'Case 3 (Detention)' in the plans.

The designer shall determine whether tight soil or other conditions at each individual inlet indicates the need for underdrain in Case 3 conversions and shall call for Underdrain, Type I in the plans.

### METHOD OF PAYMENT FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- 1. Existing inlets converted to traversable slot tops under Cases 1, 2 and 3 shall be paid for as inlets partial, each. Case shall not be included in the pay item description.
- 2. All ditch reconstruction work within 35 feet of each traversable slot conversion, whether required by these details or as a direct result of the conversion, shall be included as a part of the partial cost. Reconstruction work shall include excavation and removal of surplus materials or borrow materials in place, grading, compaction, shaping and restoration of disturbed turf. Sodding, ditch pavement and underdrain are not included as part of the inlet partial cost and are to be paid for separately.
- 3. Concrete inlet pavement and sodding shall be in accordance with the sections on this detail and with the Plan on Sheet 4 and Sections AA, BB and CC (as Case 1) and tabular quantities on Sheet 5.
- 4. Unit price and payment shall constitute full compensation for inlet conversion (including concrete inlet paving and replacement grate(s)), ditch reconstruction, restoration of disturbed turf, and shall be paid for under the contract price for Inlets (DT Bot) (Type __) (Partial), each.

Sodding shall be paid for under the contract unit price for Performance Turf, SY.

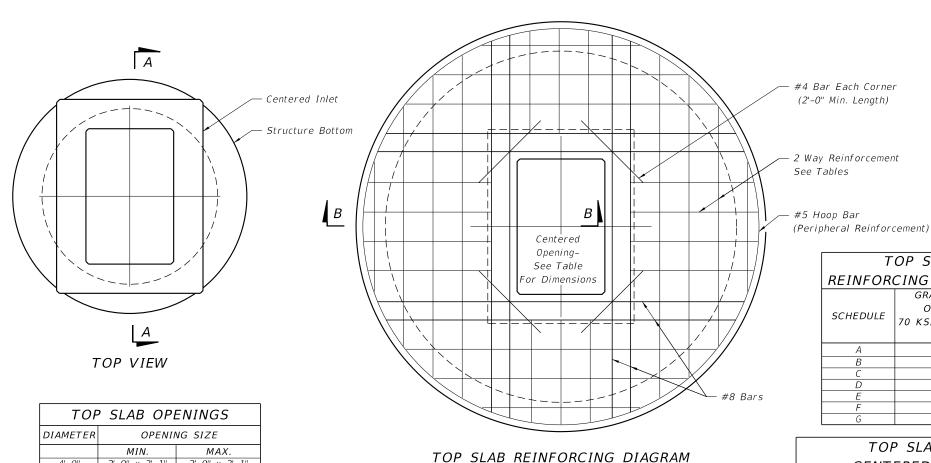
Ditch pavement shall be paid for separate from the inlet by pavement type(s) and unit(s) as called for in the plans.

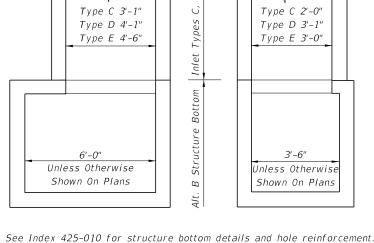
REVISION 11/01/17

DESCRIPTION:

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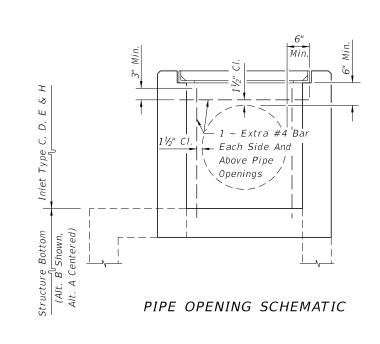


ALT. B STRUCTURE BOTTOM FOR INLETS TYPE C, D & E

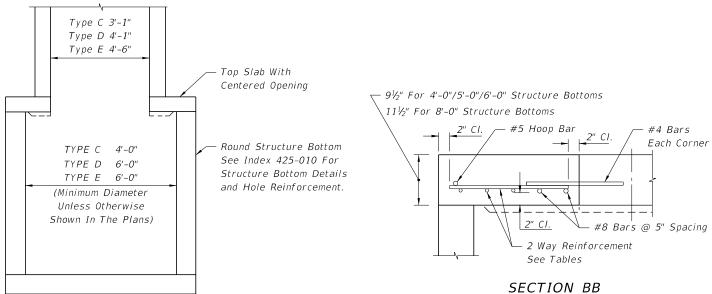
TOP SLAB			
REINFOR	REINFORCING SCHEDULE		
	GRADE 60 (BAR)		
SCHEDULE	OR 65 KSI &		
SCHEDULE	70 KSI (WIRE FABRIC)		
	In.²/ft.		
Α	0.20		
В	0.24		
С	0.37		
D	0.53		
Ε	0.73		
F	1.06		
G	1.45		

<u>&gt;</u> 0
2.5
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#### TOP SLAB WITH CENTERED OPENING REINFORCING SLAB SLAB (2 WAYS) DEPTH THICKNESS SCHEDULE SIZE: 4'-0" 0.5'-40' 91/2" SIZE: 5'-0" .5' < 30' 91/2" 80'-40' 91/2" D SIZE: 6'-0" 5' < 8' 91/5" < 18' 91/2" 3' < 30' 91/2" D 0' < 37' 91/2" 37'-40' 91/2" SIZE: 8'-0" .5' < 9' 111/2" < 15' 111/2" D 5' < 23' 111/2" 3' < 33' 111/2" 3'-40' 111/2"



2'-0" x 3'-1" 3'-1" x 4'-1" 4'-0" 2'-0" x 3'-1" 5'-0" 2'-0" x 3'-1" 2'-0" x 3'-1" 3'-0" x 4'-6" | 2'-0" x 3'-1" | 3'-0" x 4'-6"



SECTION AA

DESCRIPTION:

ALT. A STRUCTURE BOTTOM FOR INLETS TYPE C, D AND E

**REVISION** 11/01/17

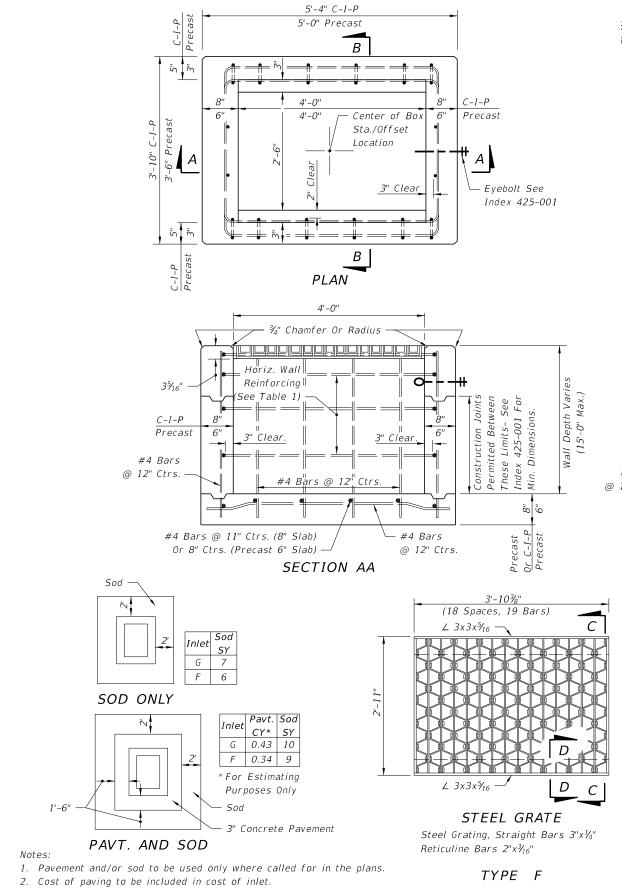
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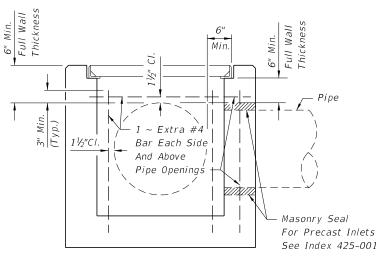
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DITCH BOTTOM INLET TYPES C, D, E AND H

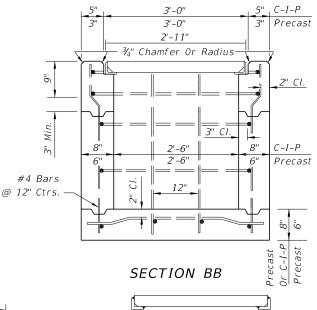
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#### (TYPE F SHOWN, TYPE G SIMILAR) PIPE OPENING SCHEMATIC



# ¾" Notch Permitted (At The Option Of The Manufacturer) ∠ 3x3x5/16

SECTION CC

# SECTION DD

# HORIZONTAL WALL REINF. SCHEDULES TYPE F INLET (TABLE 1)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in²/ft)	BARS	WWF
0' - 4'	A12	0.20	12"	8"
4' - 7'	A6	0.20	6"	5"
7' - 12'	B5.5	0.24	5½"	5"
12' - 15'	Special 1	0.267	5"	4"

#### GENERAL NOTES

- 1. These inlets are designed for use in ditches, medians, pavement areas, or other areas subject to heavy wheel loads, minimal debris, and bicycle traffic. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet. When inlet is placed in areas subject to bicycle traffic, install filler bar when clearance or gap is greater than 5%" as shown in Index 425-031.
- 2. When Alternate G grate is specified in plans, the grate is to be hot dip galvanized after fabrication.
- 3. These inlets may be used with Alternate B structure bottoms, Index 425-010. The inlet and bottom combinations are to be paid for under the contract unit price for inlets (DT Bot) (Type F (or G)) (J Bot, Depth), Ea.
- 4. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 5. For supplemental details, see Index 425-001.
- 6. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Bars to be cut or bent for  $1\frac{1}{2}$ " clearance around pipe opening. Provide one additional #4 bar above and at each side of pipe opening, as shown.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.

RECOMMENDED MAXI	MUM PIPE SIZES
INLET INSIDE WIDTH	PIPE SIZE
2'-6" (Type F)	18"
4'-0" (Type F)	30"
4'-10" / 5'-0" (Type G)	42"

Note: Recommended sizes are for concrete pipe. Sizes for other types of pipe must be verified for fit in accordance with Index 425-001. For larger pipe sizes see Note 3.

# PAVEMENT AND SODDING

DESCRIPTION: **REVISION** 11/01/17

FDOT

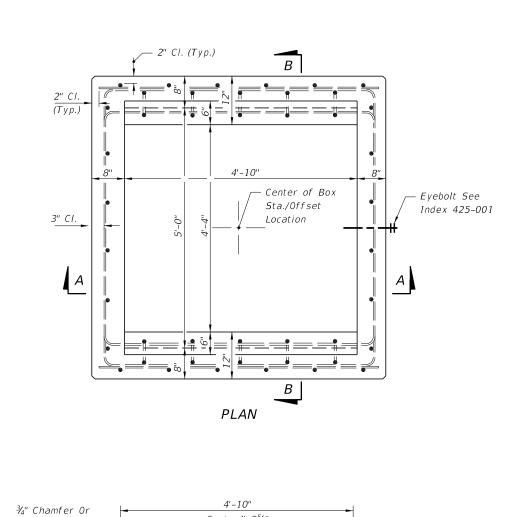
FY 2019-20 STANDARD PLANS

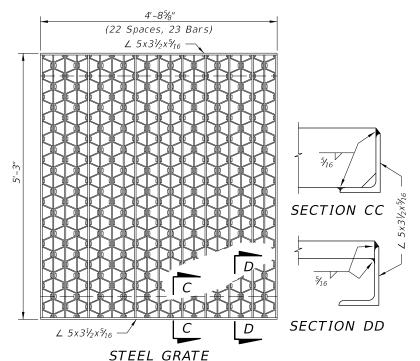
DITCH BOTTOM INLET TYPES F AND G

INDEX

SHEET

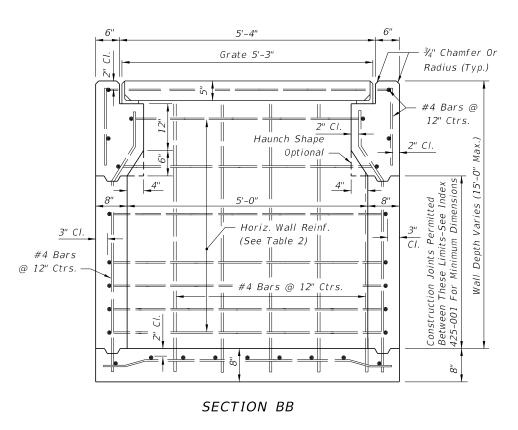
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5" Steel Decking, Weight 630 Lbs. Main Bars 5" x 1/4" Intermediate Bars  $1\frac{1}{2}$ "  $x\frac{1}{4}$ ", Reticuline Bars  $1\frac{1}{4}$ "  $x\frac{3}{16}$ "

# Grate 4'-85%" Radius (Typ.) Lifting Loop -Eyebolt See Index 425-001 3" C1. #4 Bars @ 12" Ctrs. #4 Bars @ 12" Ctrs. └─ #4 Bars @ 8" Ctrs. SECTION AA



TYPE G INLET (TABLE 2)

WALL DEPTH	SCHEDULE	AREA (in²/ft)	MAX. S BARS	PACING WWF
0' - 3'	A12	0.20	12"	8"
3' - 7'	A6	0.20	6"	5"
7' - 10'	B5.5	0.24	5½"	5"
10' - 15'	C6.5	0.37	6½"	6"

TYPEG

REVISION 11/01/17

DESCRIPTION:

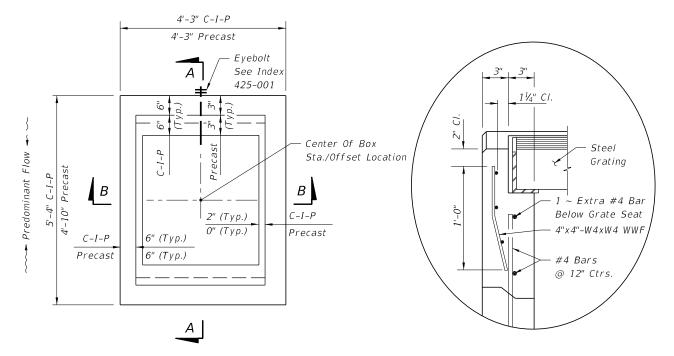
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FY 2019-20 STANDARD PLANS

DITCH BOTTOM INLET TYPES F AND G

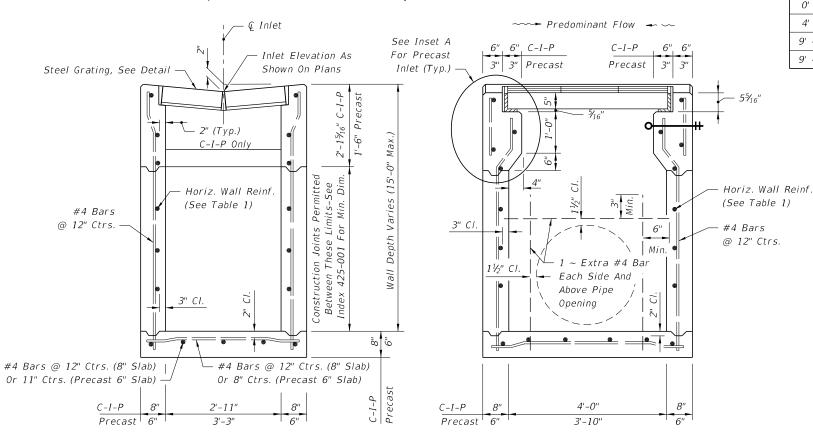
INDEX 425-053

SHEET 2 of 2



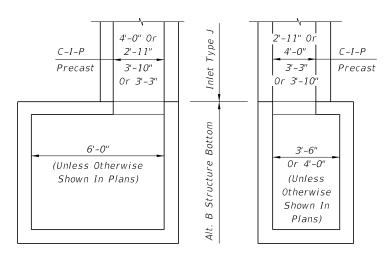
PLAN (CAST-IN-PLACE INLET SHOWN, WITHOUT GRATE, PRECAST INLET SIMILAR)

INSET A (PRECAST OPTION)



(Pipe Opening Not Shown) SECTION BB

(Pipe Opening Shown)
SECTION AA



NOTE: Alt. B Structure Bottom Only. See Index 425-010 for structure bottom details and hole reinforcement.

#### INLET WITH STRUCTURE BOTTOM

# HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(In²/ft)	BARS	WWF
0' - 4'	A12	0.20	12"	8"
4' - 9'	A6	0.20	6"	5"
9' - 12'	A4	0.20	4"	3"
9' - 15'	B5.5	0.24	5½"	5"

## RECOMMENDED MAXIMUM PIPE SIZES

INLET INSIDE WIDTH	PIPE SIZE
2'-11" or 3'-3"	24"
3'-10" or 4'-0"	<i>30</i> "

Note: Recommended sizes are for concrete pipe.
Sizes for other types of pipe must be
verified for fit in accordance with Index
425-001. For larger pipe, see Structure
Bottom detail above and Index 425-010.

#### GENERAL NOTES

- 1. This inlet is designed for use in ditches, medians, pavement areas or other areas subject to heavy wheel loads with minimal debris. This inlet is not for use in areas subject to bicycle traffic. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet.
- 2. All reinforcing Grade 60 bars with 2" min. cover unless otherwise noted. See Index 425-001 for equivalent area of welded wire fabric. Cut or bend bars out of way of pipe when necessary; bars to clear pipe by 1½".
- 3. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 4. When alternate G grate is specified in plans the grate is to be hot dip galvanized after fabrication.
- 5. For supplemental details, see Index 425-001.
- 6. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 7. Cost of ditch paving to be included in cost of inlet. Sodding to be paid for under contract unit price for Performance Turf, SY.

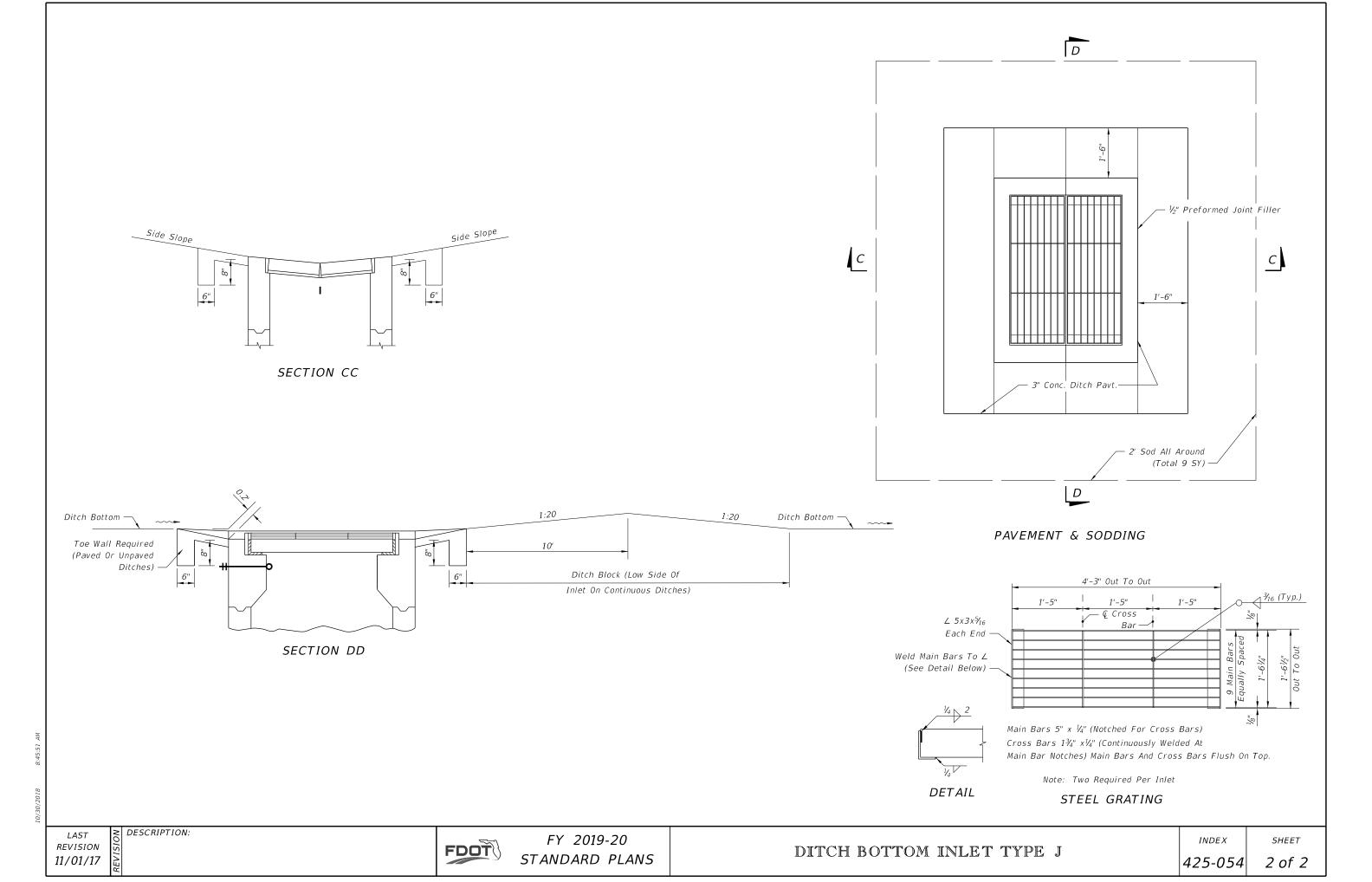
LAST REVISION 11/01/17

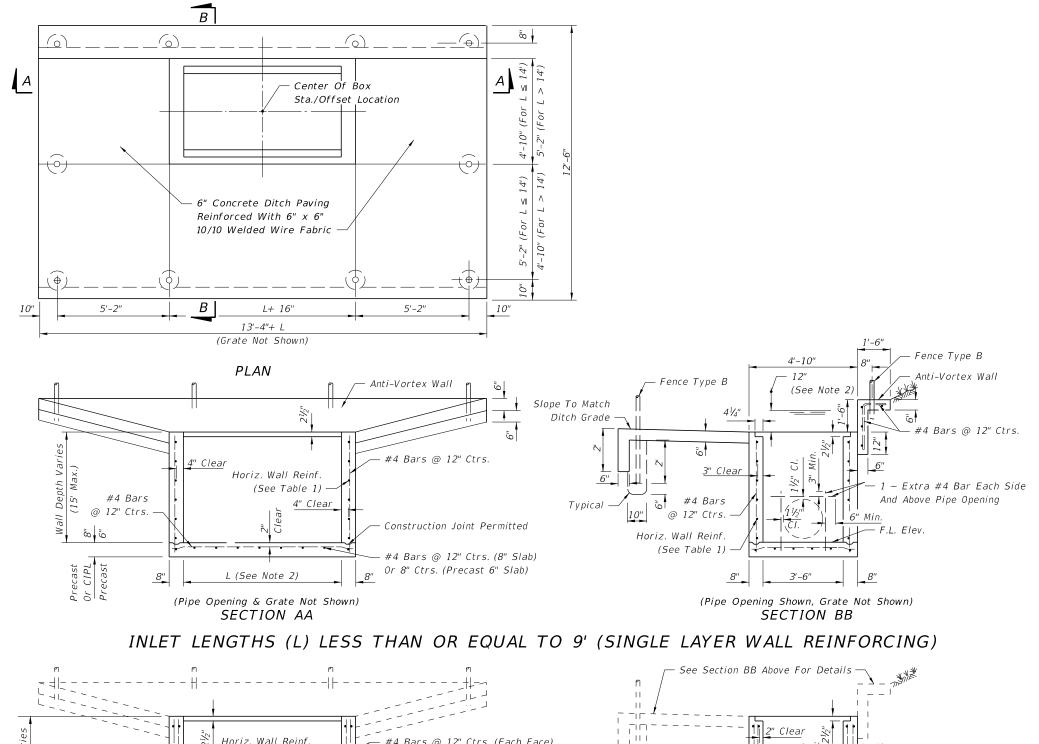
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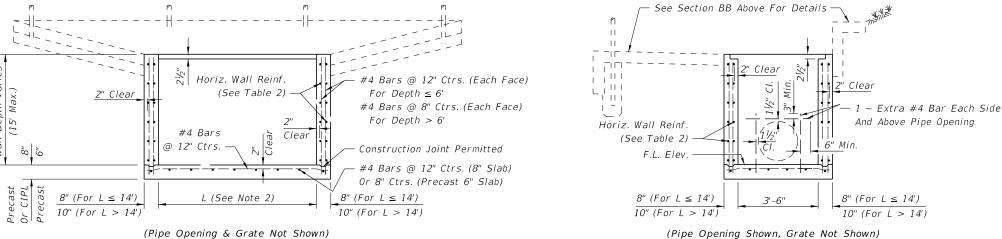
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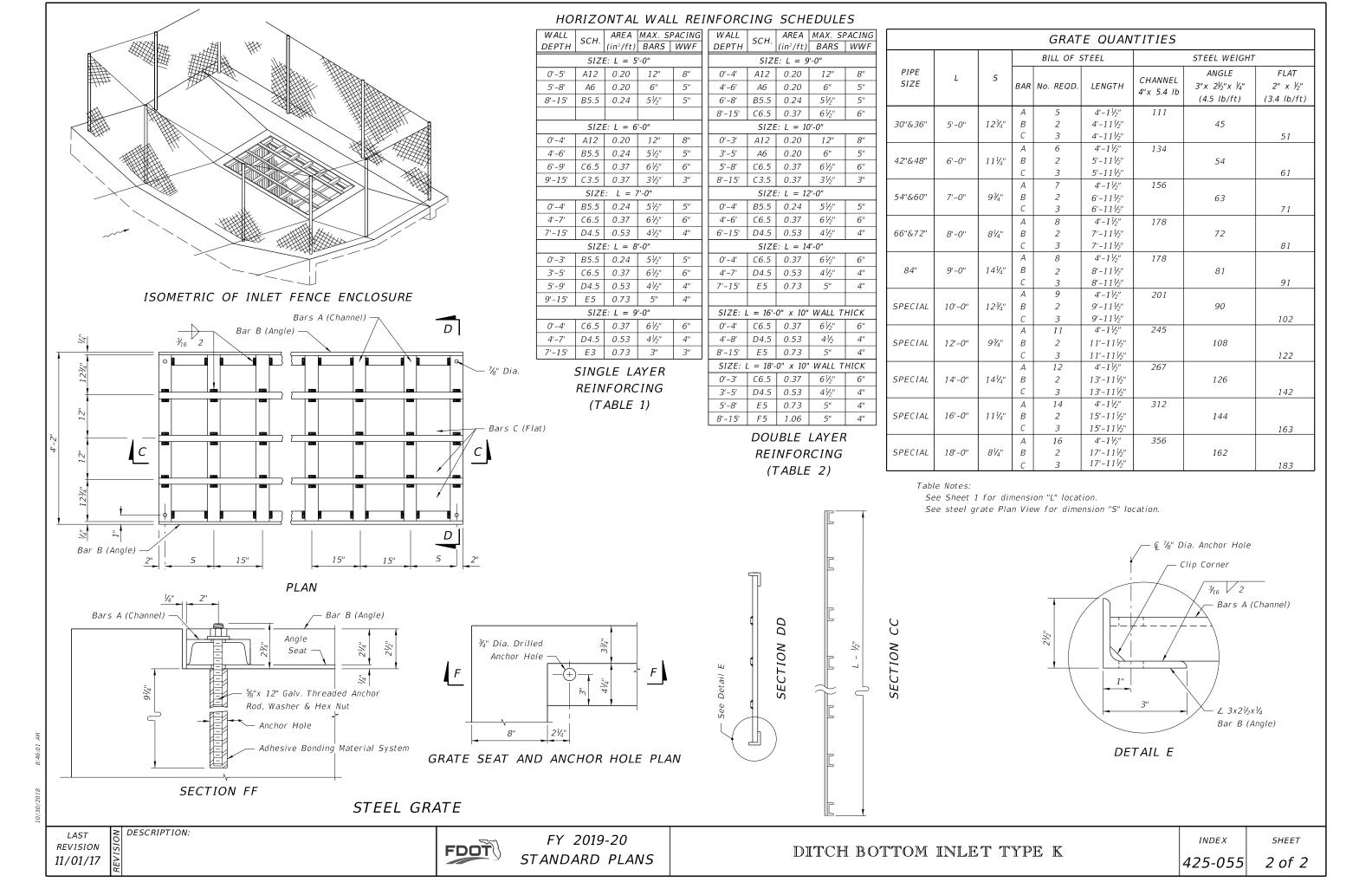
INLET LENGTHS (L) GREATER THAN OR EQUAL TO 9' (DOUBLE LAYER WALL REINFORCING)

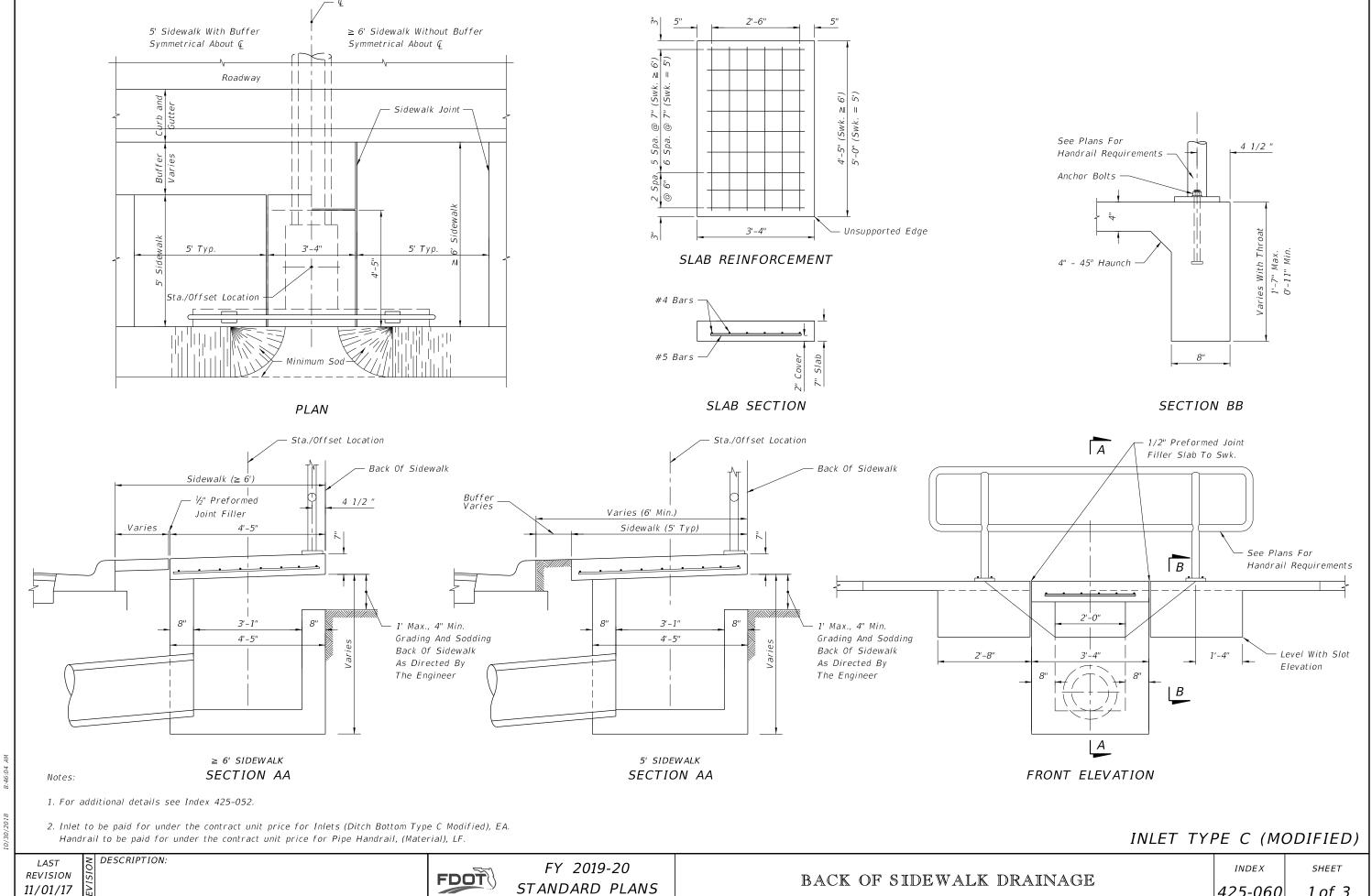
#### GENERAL NOTES

- 1. This inlet is to be used at locations having high flow rates, usually where an endwall could not be utilized without hazardous intake.
- 2. Inlet length (L) shall be set by the designer for the greater of either culvert requirement or inlet pool not to exceed 12" depth. Structures over 6 feet in depth are to be checked for flotation by the designer of project drainage.
- 3. This inlet is not intended for use with Index 425-010 structure bottoms.
- 4. All exposed edges and corners shall be  $\frac{3}{4}$ " chamfer or tooled to  $\frac{1}{4}$ " radius.
- 5. Inlet and anti-vortex wall to be Class II Concrete.
- 6. All reinforcing is Grade 60 with 2" min. cover unless otherwise noted. See Index 425-001 for equivalent area of welded wire fabric (WWF). Bars to be cut or bent for  $1\frac{1}{2}$ " clearance around pipe opening. Bend top and corner bars to clear anchor holes.
- 7. Channel section C 3x6 at 14" max. bar spacing may be used as an alternate for the C 4x5.4 channel at 15" bar spacing.
- 8. Channels and bars for grate shall be ASTM A242/A242M, A572/A572M or A588/A588M, Grade 50 steel, and galvanized in accordance with Specification Section 975.
- 9. Fence enclosure shall be Fence Type B (Index 550-002). All posts to be set in concrete. A minimum of 10 posts required. Corner and approach side posts to be 3" nominal diameter.
- 10. Cost of ditch paving, anti-vortex wall, grate, concrete, reinforcing steel and fence enclosure to be included in the cost of inlet. Inlet to be paid for under the contract unit price for Inlets (DT Bot) (Type K), Each.
- 11. Anchor Bolts shall be ASTM F1554 Grade 36 fully threaded headless bolts, installed in accordance with Specification Sections 416 and 937. Nuts shall be ASTM A563 or A194 and washers shall be ASTM F436 or Type A plain washers. All nuts, bolts and washers shall be galvanized.

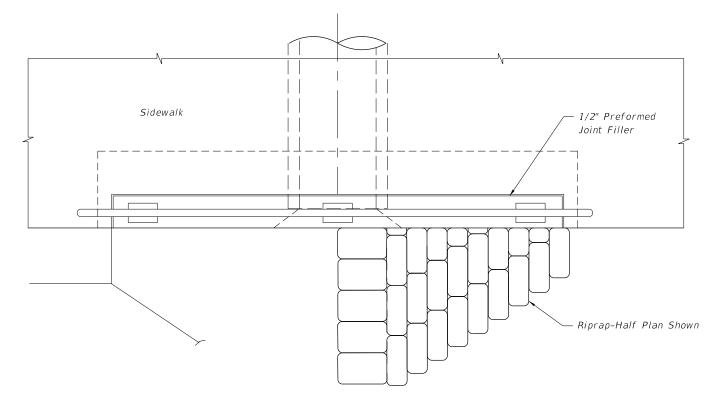
SECTION AA

SECTION BB

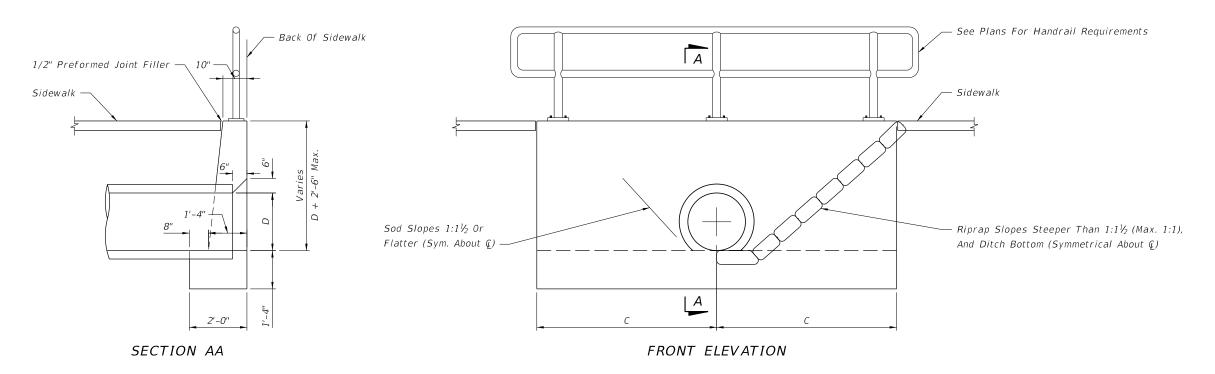




FDOT



PLAN



1. Maximum pipe size shall be 24" diameter.

≥ DESCRIPTION:

- 2. Grading back of sidewalk varies and shall be done as directed by the Engineer.
- 3. Concrete quantities shown are for maximum wall heights, and shall be basis for estimate and payment.
- 4. Riprap quantities shown are for estimate purposes only. Cost of riprap to be included in cost of the endwall.
- 5. Endwalls to be paid for under the contract unit price for Concrete Class I (Endwalls), CY. Handrail to be paid for under the contract unit price for Pipe Handrail, (Material), LF.

Pipe Size (in)	С	Concrete Class I (CY)	Sand-Cement Riprap (CY)
15	4'-9"	2.3	1.1
18	5'-3"	2.6	1.3
24	6'-3"	3.3	1.8

SPECIAL CONCRETE ENDWALL

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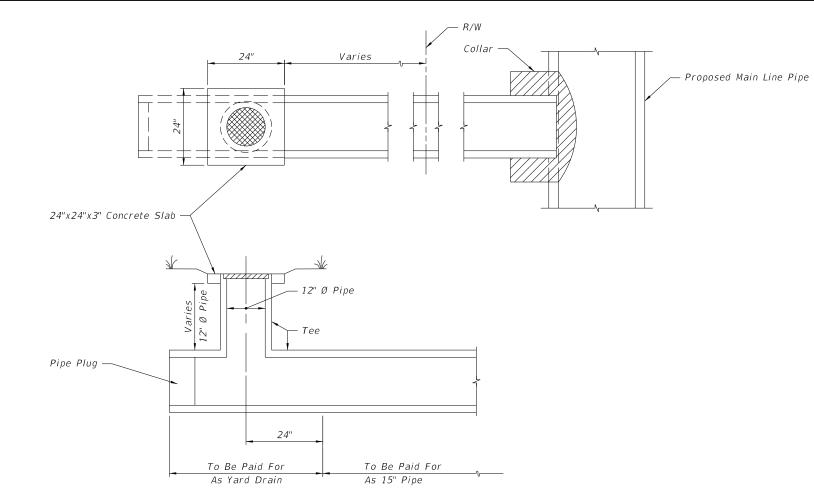
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FY 2019-20 STANDARD PLANS

BACK OF SIDEWALK DRAINAGE

INDEX 425-060

SHEET 2 of 3



#### YARD DRAIN ITEM INCLUDES:

- 1. 15" x 15" x 12" Concrete or PVC Tee 4' long.
- 2. Grate diameter =  $14-\frac{1}{4}$ " Thickness =  $2-\frac{1}{2}$ " Flow area = 45 sq in min.Light Duty Cast Iron, see Specification Section
- 3. 12" pipe as necessary.
- 4. 0.04 Cubic yards concrete for slab.

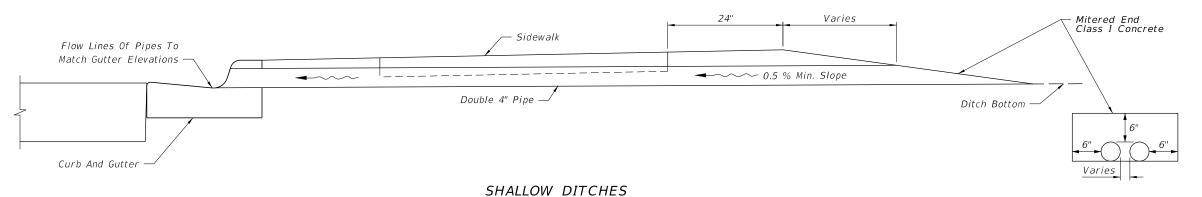
Note:

Miter to slope.

#### YARD DRAINS

#### Notes:

- 1. Yard drains to be located outside the R/W. Drainage area should not exceed 750 SF (grate flow 0.1 Cfs).
- 2. Yard drains may be constructed at the option of the property owner as shown on the plans.
- 3. Cost of plugs and collars to be included in the cost for 15" pipe. For collar and plug details see Index 430-001.
- 4. Yard drains to be paid for under the contract unit price for Yard Drains, EA.



#### Notes:

- 1. To be constructed at locations as directed by the Engineer.
- 2. Either cast iron pipe or PVC rigid conduit, U.L. listed for direct sunlight exposure, Schedule 40, may be used.
- 3. Pipe and Mitered End to be paid for under the contract unit price for either Cast Iron Soil Pipe (Standard) (4"), LF or PVC Pipe For Back Of Sidewalk Drainage (4"), LF.

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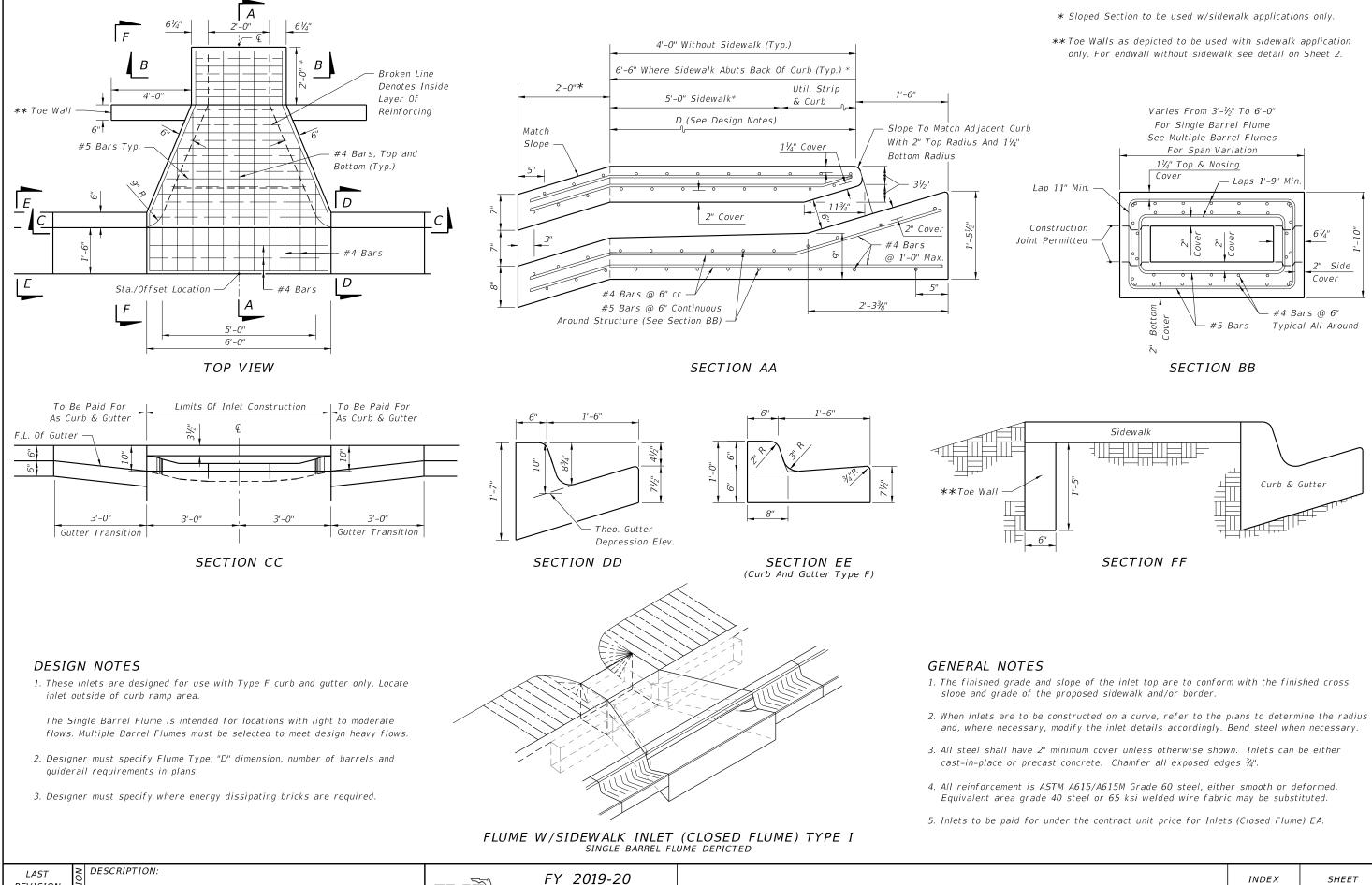
DESCRIPTION:

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BACK OF SIDEWALK DRAINAGE

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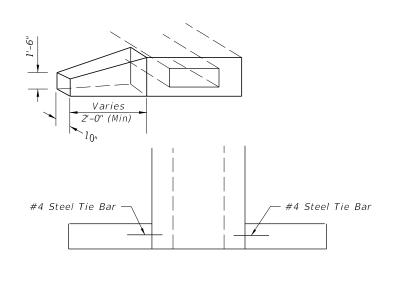


STANDARD PLANS

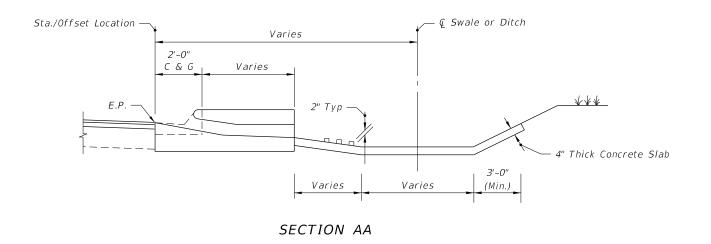
CLOSED FLUME INLET

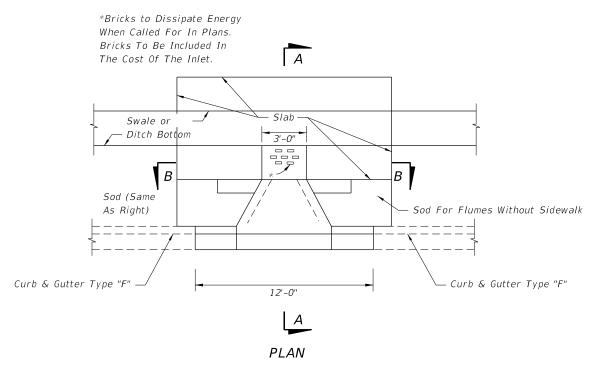
425-061

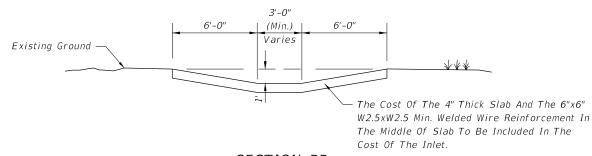
1 of 3



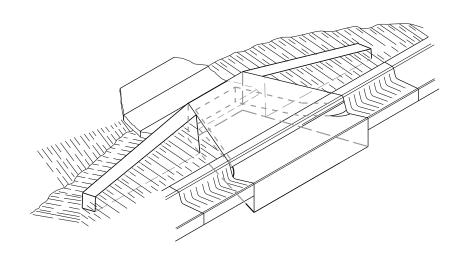
#### **ENDWALL**







SECTION BB



FLUME W/O SIDEWALK INLET (CLOSED FLUME) TYPE II SINGLE BARREL FLUME DEPICTED

REVISION 11/01/17

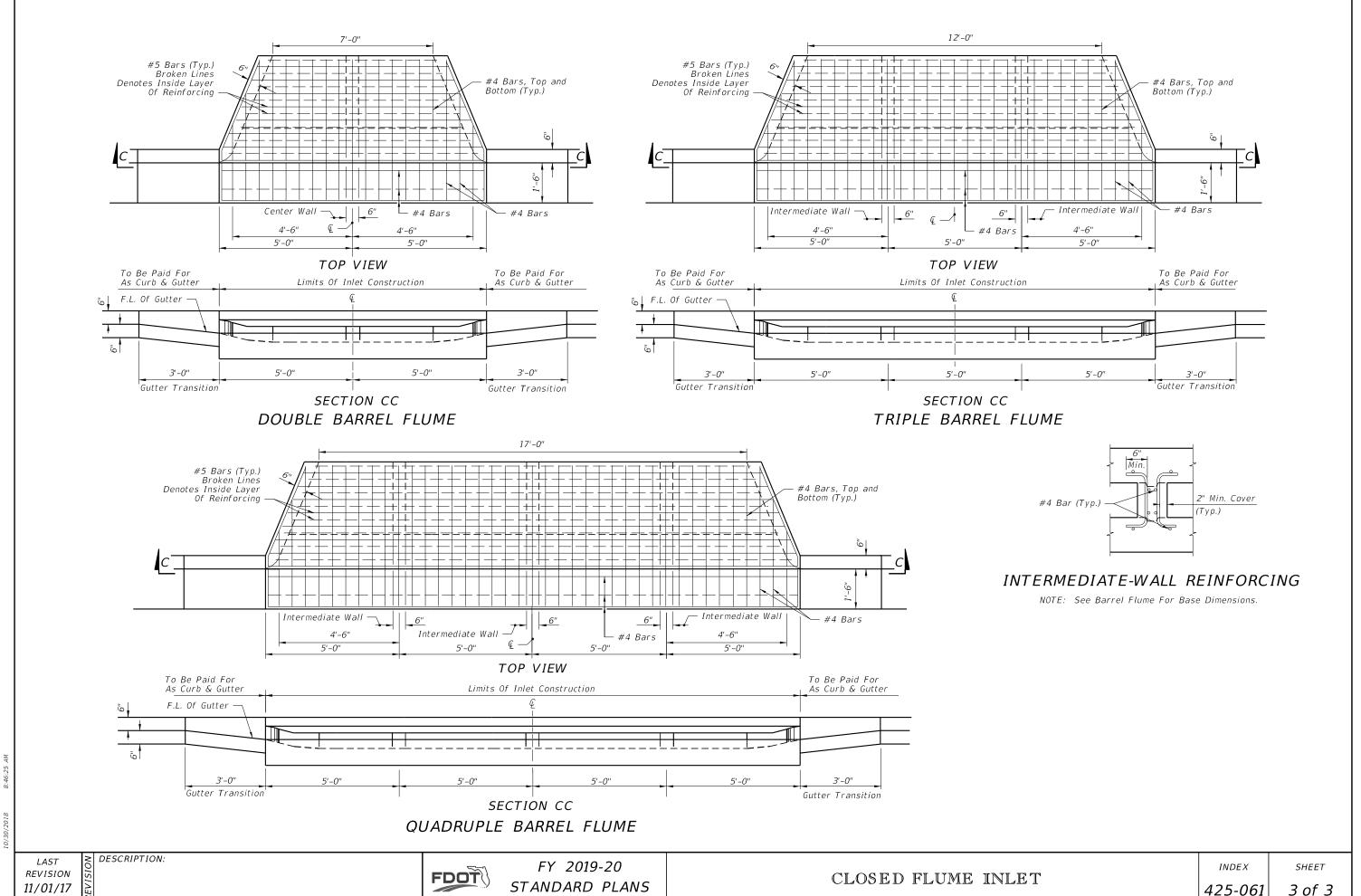
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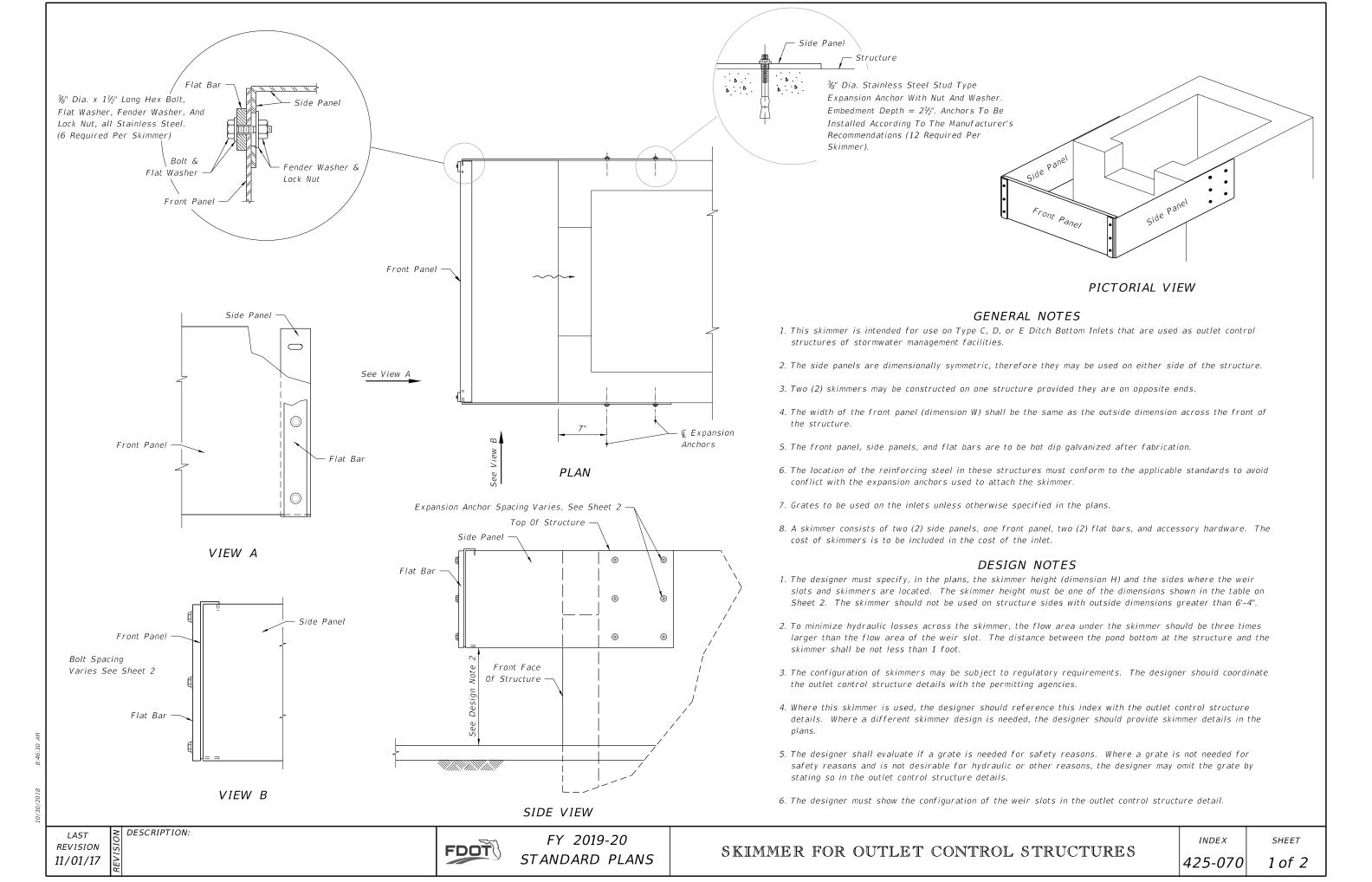
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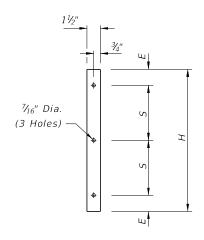
CLOSED FLUME INLET

INDEX 425-061 SHEET



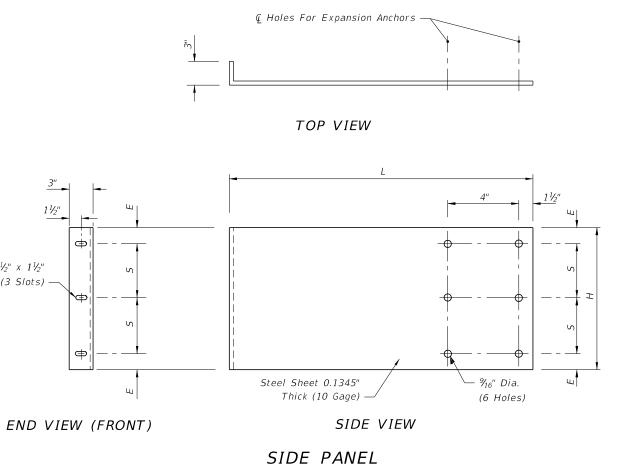


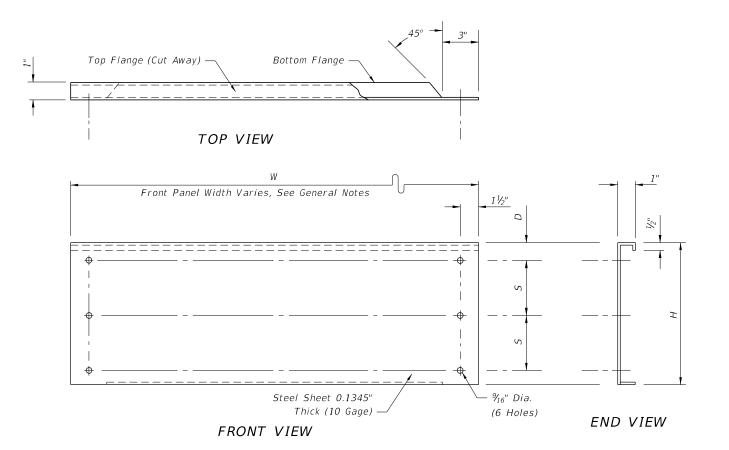
DIMENSIONS				
Skimmer Height as Specified in the Plans				Bolt Spacing
Н	D	E	L	5
	•	Inches		
12	3 ¾ ₁₆	3	28	3
14	3 ¾ ₆	3	28	4
16	3 ¾ ₁₆	3	28	5
18	3 ¾ ₁₆	3	28	6
20	4 ¾ ₁₆	4	31	6
22	4 ¾ ₁₆	4	31	7
24	4 ¾6	4	31	8
26	4 ¾ ₁₆	4	31	9
28	4 ¾ ₁₆	4	31	10
30	5 ¾ ₁₆	5	31	10
32	5 ¾ ₁₆	5	31	11
34	5 ¾ ₁₆	5	31	12
36	6 ¾6	6	31	12
38	6 ¾ ₆	6	31	13
40	6 ¾6	6	31	14



½" Thick x 1½" Wide

FLAT BAR





FRONT PANEL

≥ DESCRIPTION: LAST REVISION 11/01/17

1 1/2"

½" x 1½" (3 Slots) —

FDOT

FY 2019-20 STANDARD PLANS

SKIMMER FOR OUTLET CONTROL STRUCTURES

INDEX 425-070

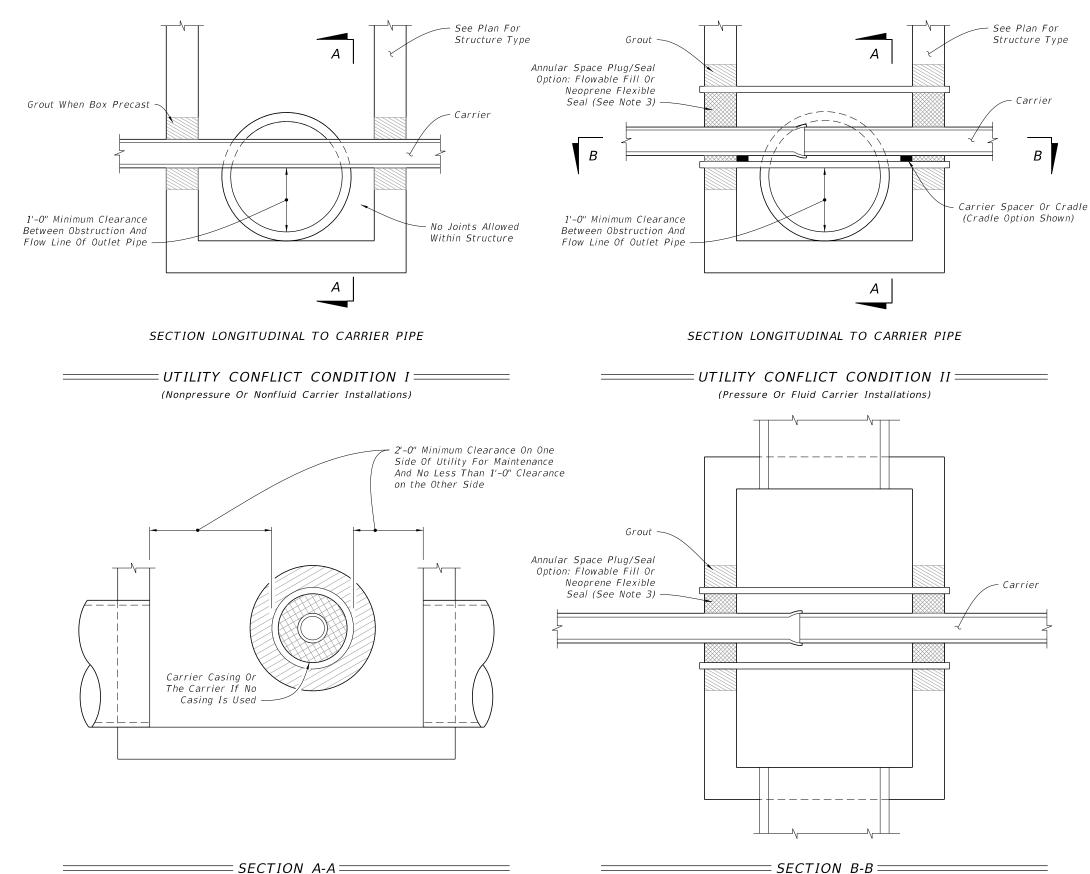
SHEET 2 of 2

## NOTES:

- 1. These details are for construction field expediency to resolve utility conflicts that cannot be remedied by relocation. For conflicts determined during design, use the construction shop drawings for structure details.
- 2. Concrete used in conflict structures shall be as specified in ASTM C478. 4000 psi may be used in lieu of Class I concrete.
- 3. Maximum opening for pipe shall be the pipe OD plus 6". Mortar used to seal the pipe into the opening will be of such mix that shrinkage will not cause leakage into or out of the structure.
- 4. If the conflict structure is round or there are multiple inlet or outlet pipes, then the wall section should be reviewed for strength.
- 5. If during construction or the plans design process it is determined that a potable water supply line must pass though a storm drain structure, it must be in compliance with Chapter 62-555.314 (3) F.A.C. and shown on the design or construction plans and submitted to the Florida Department of Environmental Protection (FDEP) Administrator For Drinking Water in the respective FDEP District for review and comment. This index and rule citation provide accepted methods for addressing conflicts when and where they cannot be reasonably avoided. To be submitted along with the plans shall be a justification describing inordinate cost and the impracticality of avoidance. If identified, properly justified, and accomplished in accordance with this index, approval is granted. Upon request, the Utility Agency Owner (UAO) must provide support data on the cost of relocation or adjustment to the FDOT for submittal to the FDEP. See the following web site for District FDEP Drinking Water Contacts: www.dep.state.fl.us/water/drinkingwater/index.htm and click on "Organization" on the menu to the right.

### DESIGNER'S NOTES:

"Sumped" conflict manholes shall not be used unless the system is hydraulically designed to account for the headloss generated if the sump is completely blocked

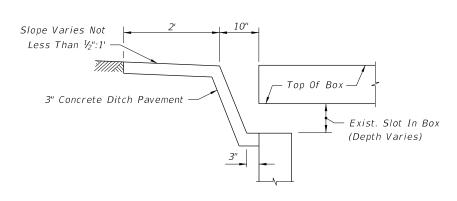


**REVISION** 11/01/17

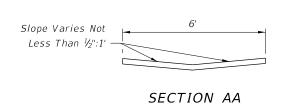
DESCRIPTION:

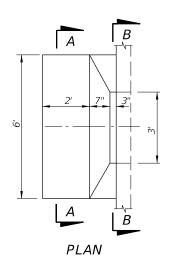


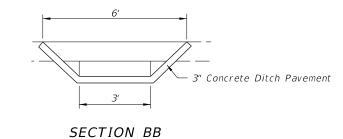
FY 2019-20 STANDARD PLANS UTILITY CONFLICT PIPES THRU STORM DRAIN STRUCTURES



## LONGITUDINAL SECTION

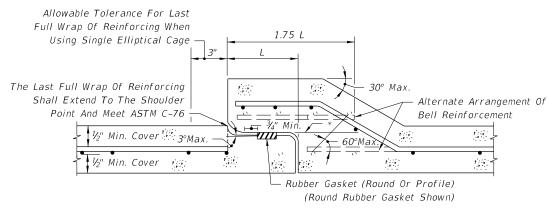






SAFETY MODIFICATION FOR INLETS IN BOX CULVERTS

Class	es II,III,IV,V	; Wall A,B,C
Nominal	Design	Maximum
Pipe	Bell	Reinforcement
Diameter	Reinforcement	Under Tolerance
Diameter	in² per foot	in² per foot
15"	0.07	0.010
18"	0.07	0.010
24"	0.09	0.010
30"	0.12	0.010
36"	0.14	0.010
42"	0.16	0.010
48"	0.19	0.011
54"	0.21	0.012
60"	0.23	0.0135
66"	0.26	0.015
7 <i>2</i> "	0.28	0.0165
78"	0.30	0.018
84"	0.33	0.0195
90"	0.35	0.021
96"	0.37	0.0225
102"	0.40	0.024
108"	0.42	0.0255



* All circumferential steel located above this line within 1.75 L is defined as bell reinforcement.

Varies

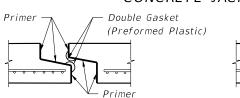
BELL AND SPIGOT

## ROUND RUBBER GASKET SHOWN DETAIL OF BELL & SPIGOT CONCRETE PIPE JOINT USING ROUND OR PROFILE RUBBER GASKET

Class NS Concrete 12" For Pipes 14"x23" Through 19"x30" Any Wire Mesh Arrangement Which 24" For Pipes 24"x38" And Larger Provides 0.126 Square Inches Of Steel Area Per Linear Foot Both Ways May Be Used; Provided The Wires Are Spaced A Minimum Of 2" And/Or A Maximum Of 6" On Centers CONCRETE JACKET

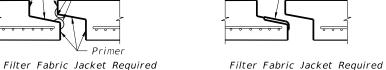
Rubber Gasket

PROFILE RUBBER GASKET



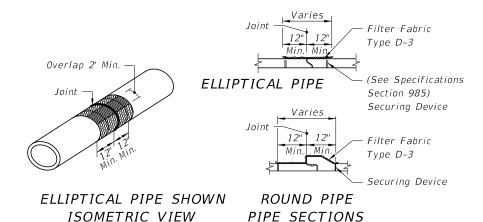
PREFORMED PLASTIC JOINT

ISOMETRIC VIEW



(BEFORE PULL-UP) (BEFORE PULL-UP) Cost of concrete jacket or filter fabric jacket to be included in cost of elliptical concrete pipe culverts.

## ELLIPTICAL CONCRETE PIPE JOINTS



Cost of filter fabric jacket to be included in cost of pipe culverts.

FOR ALL PIPE TYPES - CONCRETE PIPE SHOWN FILTER FABRIC JACKET

CONCRETE JACKET FOR CONNECTING DISSIMILAR TYPES OF PIPE AND CONCRETE PIPES WITH DISSIMILAR JOINTS

Note: Cost of concrete and bituminous coating to be included in

contract unit price for either new pipe or Mitered End Section.

Do not use a concrete jacket to join metal pipes of dissimilar

DISSIMILAR TYPES

Alternate connection must be approved by the State Drainage Engineer.

12" For Pipes 15" Thru 24"

TONGUE & GROOVE

24" For Pipes 30" And Larger

- Class NS Concrete

Note: For reinforcement see elliptical pipe concrete jacket. (All Pipe Sizes)

DISSIMILAR JOINTS

Collar Of Class NS Concrete (May Be Formed Existina By Any Method Approved By The Engineer) Proposed Existing Endwall Less Than 1' Below Grade 2-1/3" Ø Hoops  $6 - \frac{1}{2}$ " Ø x 16" Dowels Set In Adhesive Existing Endwall Bonded Material System Spigot End To Be Placed In Cut Toe Of Existing Endwall Existing Endwall Regardless To Contour Of Pipe Of Direction Of Flow

## SECTION AA

Class NS Concrete

Bituminous Coating Required For

Bituminous Material May Be Field

Extend 12" Beyond Concrete Collar

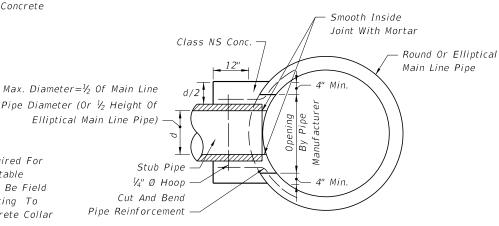
Applied) Bituminous Coating To

All Metal Pipes (Any Suitable

## LONGITUDINAL SECTION

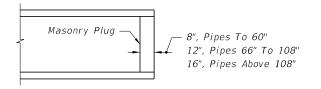
Note: Cost for removal and disposal of portions of top and toe of existing endwall and cost of concrete, reinforcing steel and construction of collar to be included in the contract unit price for pipe culvert.

# CONCRETE COLLAR FOR EXTENSION OF EXISTING PIPE CULVERTS



Cost of concrete and steel to be included in contract unit price for pipe culvert.

# CONCRETE COLLAR FOR JOINING MAINLINE PIPE AND STUB PIPE



Note: Unless otherwise called for in the plans, the cost of plugging pipes to be included in contract unit price for new pipe.

PIPE PLUG

DESCRIPTION: **REVISION** 

11/01/17

FDOT

FY 2019-20 STANDARD PLANS

materials

Class NS Concrete

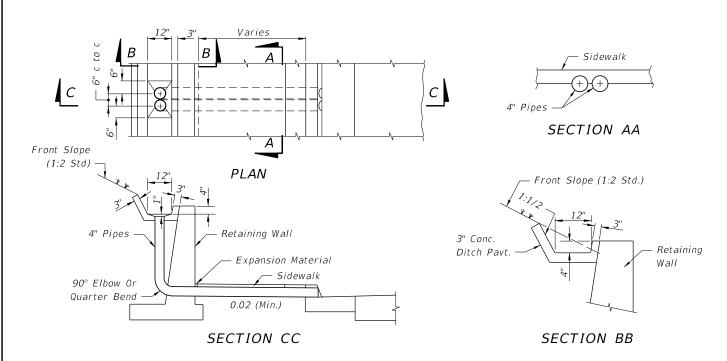
MISCELLANEOUS DRAINAGE DETAILS

INDEX

SHEET

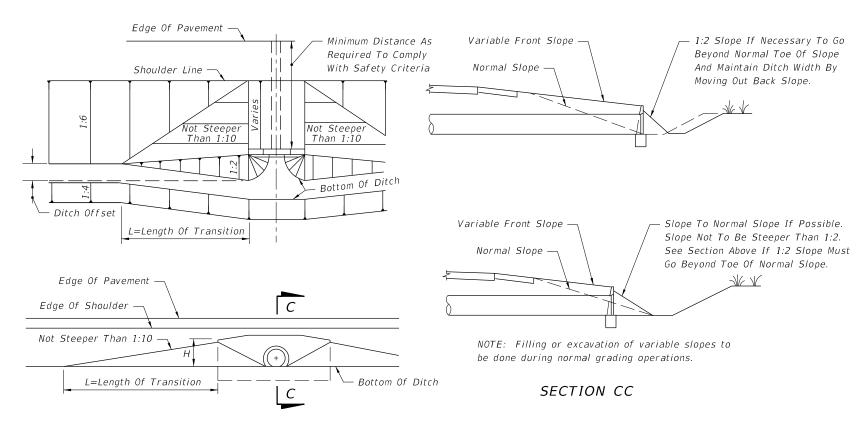
430-001

1 of 3



Note: PVC pipe, Schedule 40, to be paid for under the contract unit price for Polyvinyl Chloride Pipe Culvert (4"), LF.

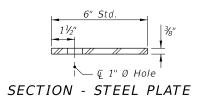
## CONCRETE GUTTER AND DRAINS AT RETAINING WALLS

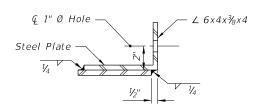


Use Larger Value Of Either:

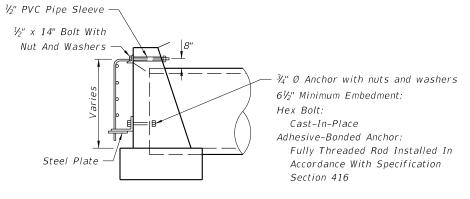
- 1. L=10xH (No Maximum)
- 2. L=10xDitch Offset (Maximum L=100')

# METHOD FOR SETTING LIMITS OF VARIABLE FRONT SLOPES AT DRAINAGE STRUCTURES

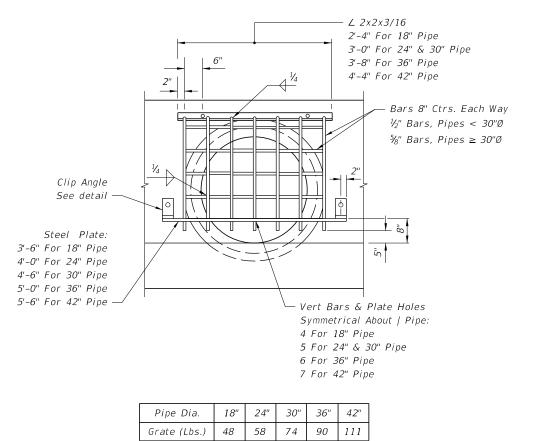




### CLIP DETAIL



### SIDE VIEW



FRONT VIEW

Note: Guards to be constructed only at locations specifically called for in plans.

## GUARD AT PIPE ENDS

**REVISION** 11/01/17

DESCRIPTION:

FDOT

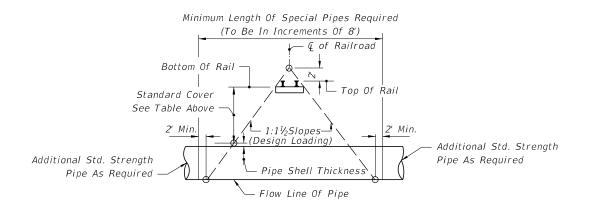
FY 2019-20 STANDARD PLANS

MISCELLANEOUS DRAINAGE DETAILS

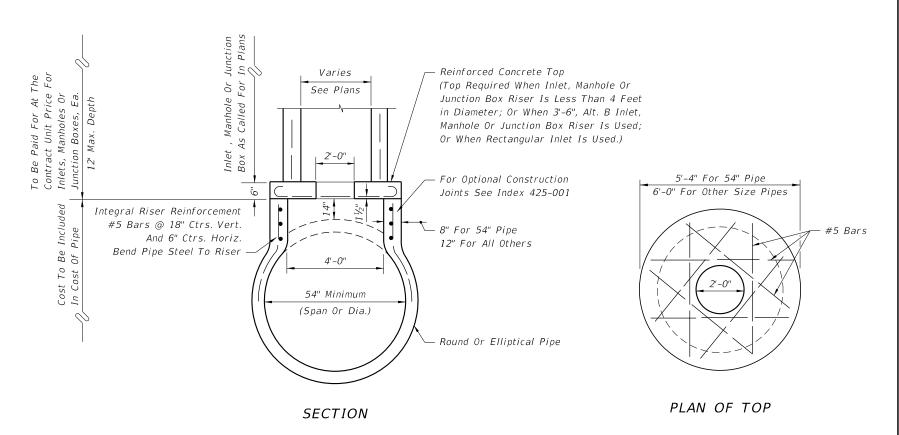
INDEX 430-001 SHEET

	CLEARANCE	STRENGTH
RAILROAD COMPANY	BELOW BOTTOM OF RAIL (FEET) ⁽²⁾	ASTM (C76) CLASS
Alabama & Gulf Coast Railway (Rail America)	5.5	IV
AN Railway & Bay Line Railroad (Genesee & Wyoming)	5.5 / 4.5 (1)	V
CSX Transportation	5.5	V
First Coast Railroad (Genesee & Wyoming)	5.5 / 4.5 (1)	V
Florida Midland, Central, and Northern Railroads (Pinsly Railroad)	5.5	V
Florida East Coast (FEC) Railway Company	5.5	IV
Florida West Coast Railroad Company	5.5	V
Georgia & Florida Railway, Inc.	5.5	V
Norfolk Southern (NS) Railway Corporation	5.5 / 4.5 (1)	V
Port of Palm Beach District Railroad	5.5	IV
Seminole Gulf Railway (LP)	6.0	V
South Central Florida Express	6.0	V
Talleyrand Terminal Railroad (Genesee & Wyoming)	5.5 / 4.5 (1)	V
South Florida Regional Transportation Authority (Tri-County Commuter Rail)	5.5	V

- (1) Distance standard for yard and industrial tracks.
- (2) Clearance is for casing pipe. All subgrade carrier pipelines and wirelines will be installed within a casing pipe which will extend from Right-of-Way line to Right-of-Way line.

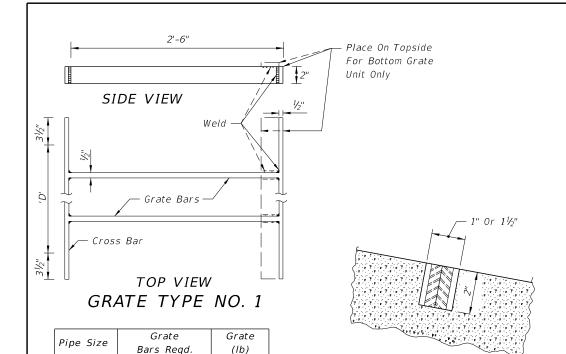


# METHOD FOR DETERMINING THE LENGTH OF SPECIAL PIPE REQUIRED UNDER RAILROADS



INLETS, MANHOLES OR JUNCTION BOXES ON INTEGRAL PRECAST CONCRETE RISER FOR CONCRETE PIPE

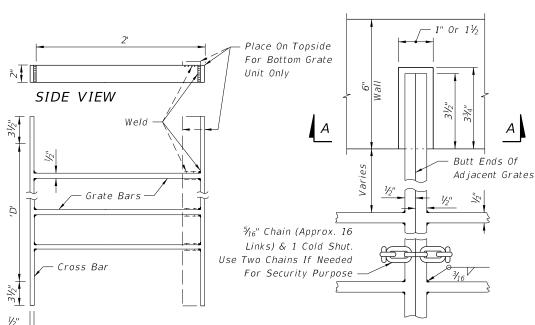
**REVISION** 11/01/17



Bars to be evenly spaced across dimension 'D' All bars 1/2" x 2"

28.93

15"



TOP VIEW GRATE TYPE NO. 2

Pipe Size	Grate	Grate
Pipe Size	Bars Reqd.	(Ib)
18"	3	33.69
24"	4	43.63
30"	5	53.55

Bars to be evenly spaced across dimension 'D'. All bars 1/2" x 2".

DESCRIPTION:

TOP VIEW GRATE, SEAT, WELD & CHAIN DETAIL

SECTION AA

- Sta./Offset Location Bar C Bar E (As Regd Bar H₄ (Às Reqd.) Bars V Bars V 4 Bars F 1' Std. Spcg. 20" (U-Bends) 6" Spcg. - Bars F SECTION BB END VIEW

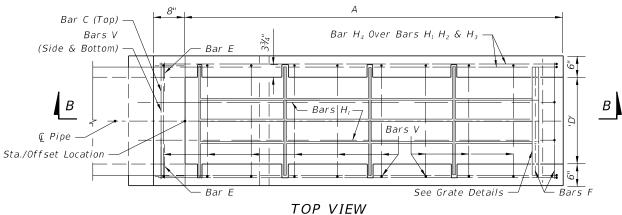


	TABLE OF DIMENSIONS AND QUANTITIES												
	Pipe Size	Α	D	Class I Conc.	Reinf. Steel	Number Of 0	Grates Reqd.	Total	Sodding (SY)	Slope T	ransition		
C1	D	A	ь	(CY)	(Ib)	Type No. 1	Type No. 2	Grate Wt. (lb)	30ddillig (31)	Offset	L		
Slope	15"	5.67'	2.38'	0.85	56	2	0	57.86	15	4.2'	42'		
1:4	18"	6.67'	1.875'	1.01	73	0	3	101.08	16	4.8'	48'		
	24"	8.67'	1.875'	1.65	97	0	4	174.52	19	5.8'	58'		
	30"	10.67'	1.875'	2.33	129	0	5	267.75	21	6.9'	69'		

### GENERAL NOTES

- 1. This endwall is to be used only in the clear zone for the drainage of medians and other areas having low design velocities and negligible debris.
- 2. Reinforcing steel: All bars are size #4. Spacings shown are center to center. Laps to be 1'-5" minimum. Cover is 2" except as noted. Square welded wire fabric (two cages max.) having an equivalent cross sectional area (0.20 sq. in.) may be substituted for bar reinforcement.
- 3. Grates shall be ASTM A242/A242M, A572/A572M or ASTM A5888/A588M, Grade 50 steel. When "Alt. G" grates are specified in the plans, grates shall be galvanized in accordance with Section 975 and 425.3.2 of the Standard Specifications.
- 4. Endwall to be paid for under the contract unit price for U-Endwall, Each. Payment shall include cost of concrete, reinforcing steel, grate, and accessories. Quantities shown are for estimating purposes only.
- 5. Sod slopes 5' each side and above endwall. Sodding to be paid for under contract unit price for Performance Turf, SY.
- 6. Precasting of this endwall will be permitted. Precast units shall conform to the dimensions shown or in accordance with approved shop drawings. Request for shop drawing approval shall be directed to the State Drainage Engineer. Use Index 425-001 for opening and grouting details.
- 7. Concrete shall be Class I except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.



FY 2019-20 STANDARD PLANS

U-TYPE CONCRETE ENDWALLS WITH GRATES 15" TO 30" PIPE

INDEX 430-010

1:6

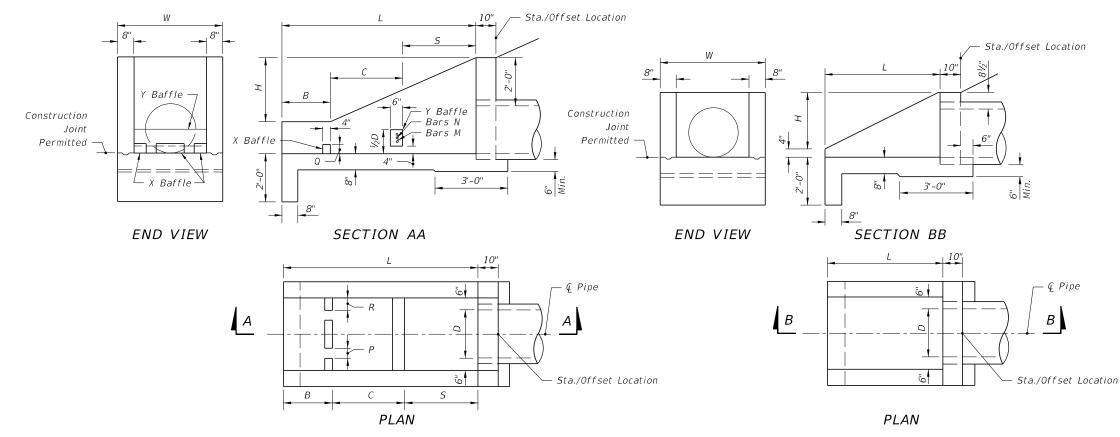
FRONT SLOPE

TRANSITION AT ENDWALL

SHEET 1 of 1

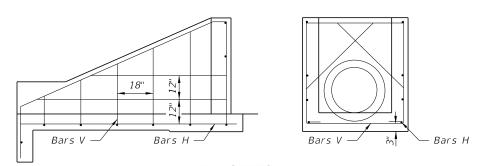
**REVISION** 

11/01/17

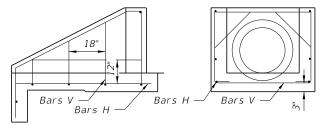


DIMENSIONAL DETAILS

DIMENSIONAL DETAILS



ALL PIPE SIZES SIDE VIEW AND BACKWALL SECTION REINFORCING DETAIL



ALL PIPE SIZES
SIDE VIEW AND BACKWALL SECTION
REINFORCING DETAIL

	DIMENSIONS AND QUANTITIES FOR ONE U-ENDWALL													
Pip	e Size Area	L	Н	W	5	В	С	Х	Baff	le	Y Ba Reinf.		Class I Conc.	Reinf. Steel
D	Sq. Ft.							Р	Q	R	Bar M	Bar N	Cu. Yd.	Lbs.
15"	1.23	5'-9"	2'-31/2"	3'-7"	2'-3"	1'-3"	2'-3"	4"	4"	4"	2 #4	1 #4	1.61	72
18"	1.77	6'-6"	2'-5"	3'-10"	2'-6"	1'-6"	2'-6"	4"	4"	5"	3 #4	2 #4	1.89	86
24"	3.14	8'-0"	2'-8"	4'-4"	3'-0"	2'-0"	3'-0"	5"	5"	6"	4 #4	3 #4	2.52	108
30"	4.91	9'-6"	2'-11"	4'-10"	3'-6"	2'-6"	3'-6"	5"	5"	7"	4 #4	4 #4	3.34	131

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DIMENS	IONS A	AND QU	'ANTITIE	S FOR (	ONE U-EN	<i>IDW ALL</i>
Pipe	Size				Class I	Reinf.
D	Area Sq. Ft.	L	Н	W	Conc. Cu. Yd.	Steel Lbs.
15"	1.23	3'-3"	1'-7½"	3'-7"	0.89	39
18"	1.77	3'-9"	1'-10 ¹ / ₂ "	3'-10"	1.05	43
24"	3.14	4'-9"	2'-41/2"	4'-4"	1.40	55
30"	4.91	5'-9"	2'-101/2"	4'-10"	1.88	64

WITHOUT BAFFLES

## **ENDWALLS FOR 1:2 SLOPES**

### GENERAL NOTES

- 1. Baffles to be constructed only when called for in plans.
- When steel grating is required on endwall see Sheet 3 of 3 for details.
- 3. All reinforcing No. 4 bars with 2" clearance except as noted.
- 4. All angles, channels and bars shall be ASTM A242/A242M, A572/A572M or A588/A588M Grade 50 steel. When designated Alternate G in the plans galvanize in accordance with Section 975 and 425-3.2 of the Standard Specifications.
- 5. Channel section C 3x6 may be substituted for C 4x5.4 channel.
- 6. Precasting of this endwall will be permitted. Precast units shall conform to the dimensions shown or in accordance with approved shop drawings. Request for shop drawing approval shall be directed to the State Drainage Engineer. Use Index 425-001 for opening and grouting details.
- 7. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 8. Sodding shall be in accordance with Index 524-001, and paid for under the contract unit price for Performance Turf, SY.
- Endwall to be paid for under the contract unit price for U-Endwall, Each. Payment shall include cost of concrete, reinforcing steel, and when called for in the plans, steel grating, baffles and accessories. Quantities shown are for estimating purposes only.

LAST REVISION 11/01/17

DESCRIPTION:

F

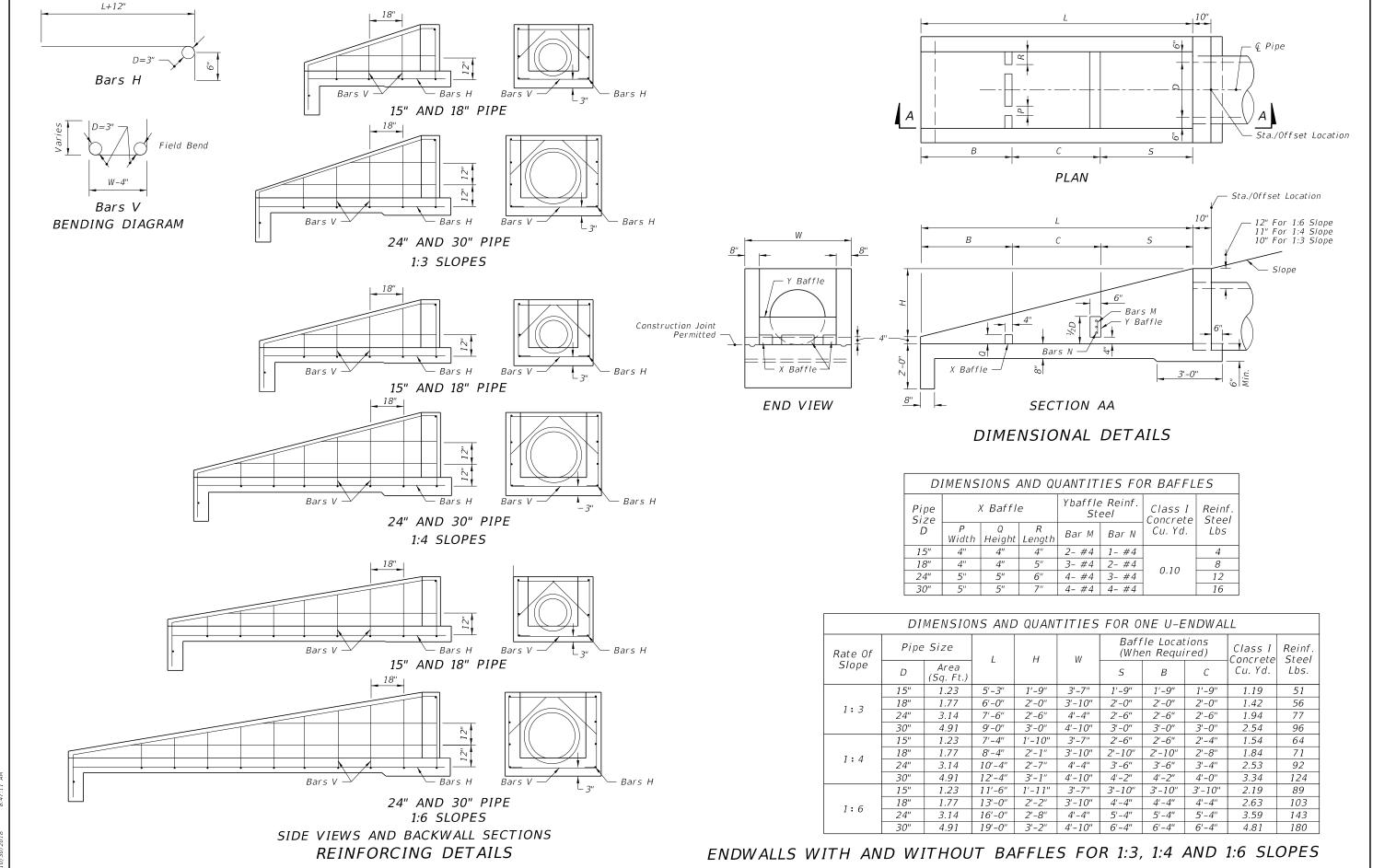
FDOT

FY 2019-20 STANDARD PLANS U-TYPE CONCRETE ENDWALLS BAFFLES & GRATE OPTIONAL 15" TO 30" PIPE

INDEX

SHEET

430-011 1 of 3



**REVISION** 

11/01/17

DESCRIPTION:

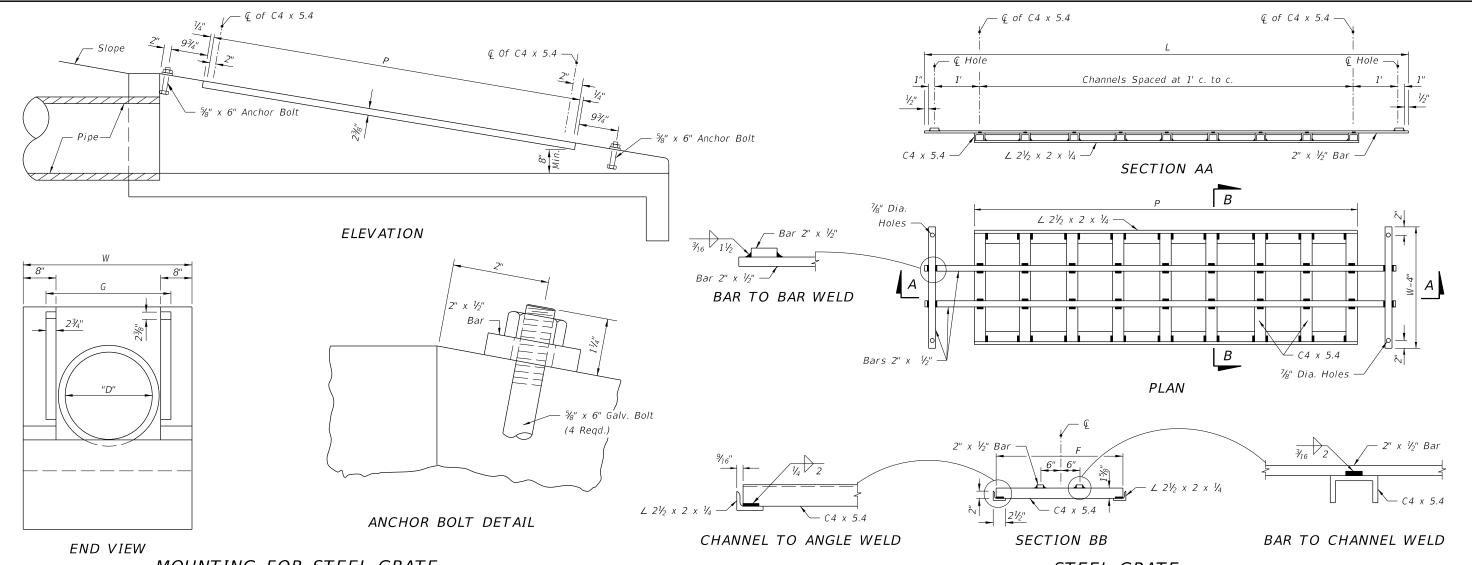
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FY 2019-20 STANDARD PLANS

U-TYPE CONCRETE ENDWALLS BAFFLES & GRATE OPTIONAL 15" TO 30" PIPE

INDEX 430-011

SHEET 2 of 3



# MOUNTING FOR STEEL GRATE

## STEEL GRATING USE CRITERIA

- 1. Provide positive debris control at all upgradient openings. Do not install grates unless one or more of the following conditions exist:
- A. Pipe culvert endwalls are located within the designated clear zone.
- B. Drainage area to culvert consists of median or infield areas or areas where debris and/or drift is negligible.
- C. Runoff to culvert is by sheet flow or in such ill defined channels that debris transport is not considered a major problem.
- D. Runoff to culvert is minor except on an infrequent basis (10 to 15 year frequency); for example a drainage basin in flat sandy terrain with normally low ground water table.
- E. Areas where culvert blockage with resultant backwater would not seriously affect roadway embankment, traffic operation or upland property.
- 2. Steel grating to be used only where called for in plans.

DESCRIPTION:

## STEEL GRATE

	7	ABLE OF	F DIME	NSIONS	S AND	QUANT	ITIES F	OR ON	IE GRA	TE	
Rate Of	Size Pipe	G	2 E	ach Bars 3.4 lb/ft	@	(X)	Channels 5.4 lb/ft	@	_	ıles @ lb/ft	Total Weight
Slope	D		L	W-4"	Ib	(X)	F	lb	Р	lb	(Ib)
	15"	2' -81/2"	9'-3"	3'-3"	85	8	2' -61/8"	111	7'-4"	53	249
1:6	18"	2' -111/2"	10'-3"	3'-6"	94	9	2' -97/8"	137	8'-4"	62	292
	24"	3' -5 ¹ / ₂ "	13'-3"	4'-0"	117	12	3' -37/8"	215	11'-4"	82	414
	30"	3' -11½"	16'-3"	4'-6"	141	15	3' -97/8"	310	14'-4"	104	555
	15"	2' -8½"	6'-3"	3'-3"	65	5	2' -67/8"	70	4'-4''	32	167
1:4	18"	2' -111/2"	7'-3"	3'-6"	73	6	2' -97/8"	92	5'-4"	39	204
	24"	3' -5½"	9'-3"	4'-0"	90	8	3' -37/8"	144	7'-4"	53	287
	30"	3' -11½"	11'-3"	4'-6"	107	10	3' -9 ⁷ / ₈ "	206	9'-4"	68	381
	15"	2' -81/2"	4'-3"	3'-3"	51	3	2' -61/8"	42	2'-4"	17	110
1:3	18"	2' -1111/2"	5'-3"	3'-6"	60	4	2' -97/8"	61	3'-4"	24	145
	24"	3' -5 ¹ / ₂ "	6'-3"	4'-0"	70	5	3' -37/8"	90	4'-4"	31	191
	30"	3' -111/2"	8'-3"	4'-6"	87	7	3' -9 ⁷ / ₈ "	145	6'-4"	46	278

**REVISION** 11/01/17

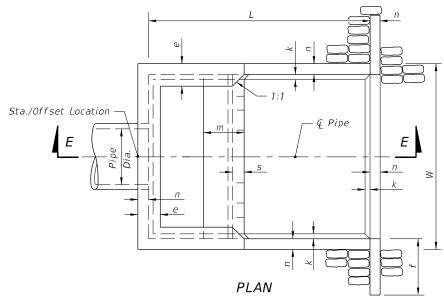
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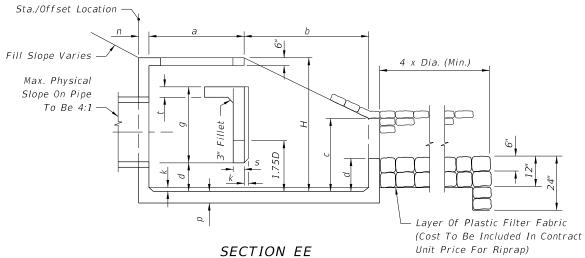
FY 2019-20 STANDARD PLANS

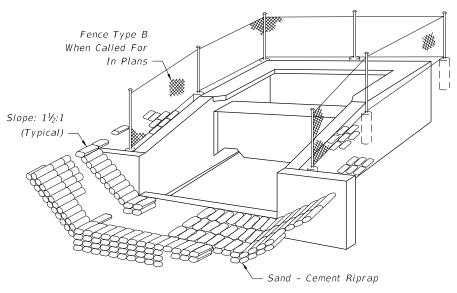
U-TYPE CONCRETE ENDWALLS BAFFLES & GRATE OPTIONAL 15" TO 30" PIPE

INDEX 430-011

SHEET





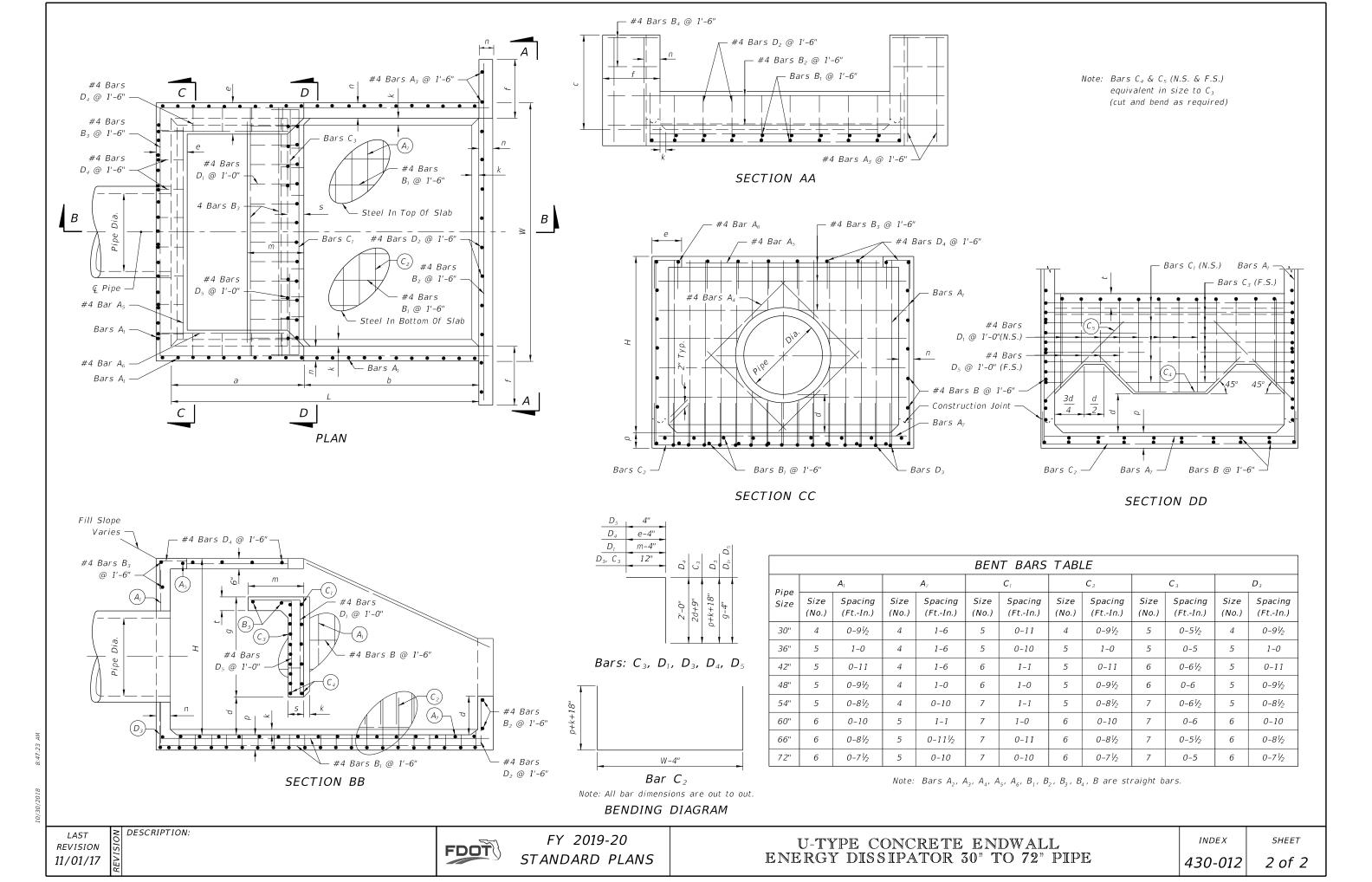


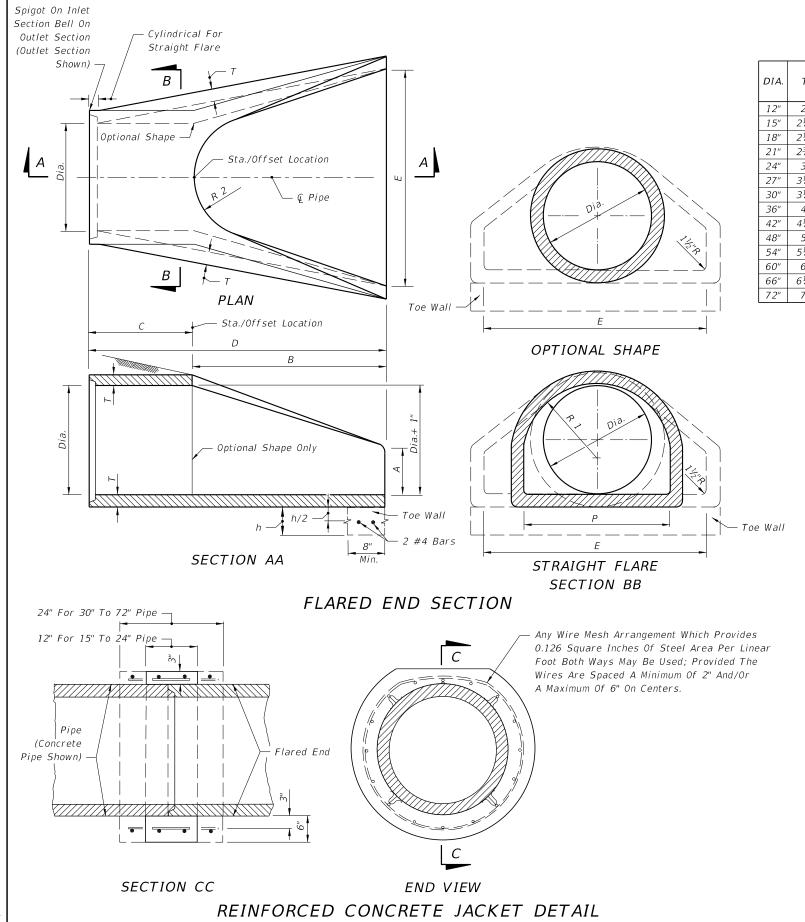
## PERSPECTIVE

## GENERAL NOTES

- 1. U-type concrete endwall energy dissipators are intended for use outside the clear zone.
- 2. Chamfer all exposed edges ¾".
- 3. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 4. Reinforcing steel shall have 2" min. cover.
- 5. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB. Riprap to be paid for under the contract unit price for Riprap (Sand-Cement) (Roadway), CY. Cost of plastic filter fabric to be included in the contract unit price for riprap.
- 6. Fencing, when called for in the plans, to be paid for under the contract unit price for Fencing, Type B, LF. See Index 550-002 for details of Type B fencing.

Pipe	e Size	0							Di	mensio	ns								Concrete	Reinf.	Sand-Cement
Dia.	Area	(Max)					Fee	t - Inc	hes						I	nche	5		Class I	Steel	Riprap (Nom.)
(in)	(SF)	(cfs)	W	Н	L	a	b	с	d	е	f	g	m	n	р	S	t	k	(CY)	(Ib)	(CY)
30	4.91	59	9-0	6-3	10-8	4-7	6-1	3-4	1-4	1-2	2-6	3-0	1-11	6	61/2	7	7	3	6.72	736	10.6
36	7.07	85	10-5	7-3	12-4	5-3	7-1	3-10	1-7	1-3	3-0	3-6	2-3	7	71/2	8	8	3	10.34	1,072	13.6
42	9.62	115	11-10	8-0	14-0	6-0	8-0	4-5	1-9	1-6	3-0	3-11	2-6	8	8½	9	8	4	14.82	1,429	17.5
48	12.57	151	13-3	9-0	15-8	6-9	8-11	4-11	2-0	1-7	3-0	4-5	2-10	9	91/2	10	8	4	20.36	2,000	22.1
54	15.90	191	14-8	9-9	17-4	7-4	10-0	5-5	2-2	1-10	3-0	4-11	3-0	10	101/2	10	8	4	27.19	2,659	27.2
60	13.63	236	16-1	10-9	19-0	8-0	11-0	5-11	2-5	1-11	3-0	5-4	3-4	11	111/2	11	8	6	34.49	3,552	32.5
66	23.76	285	17-3	11-6	20-6	8-8	11-10	6-5	2-7	2-1	3-0	5-9	3-7	12	121/2	12	8	6	42.82	4,472	38.3
72	28.27	339	18-6	12-3	22-0	9-3	12-9	6-11	2-9	2-3	3-0	6-2	3-9	12	12½	12	8	6	50.68	5,426	44.5



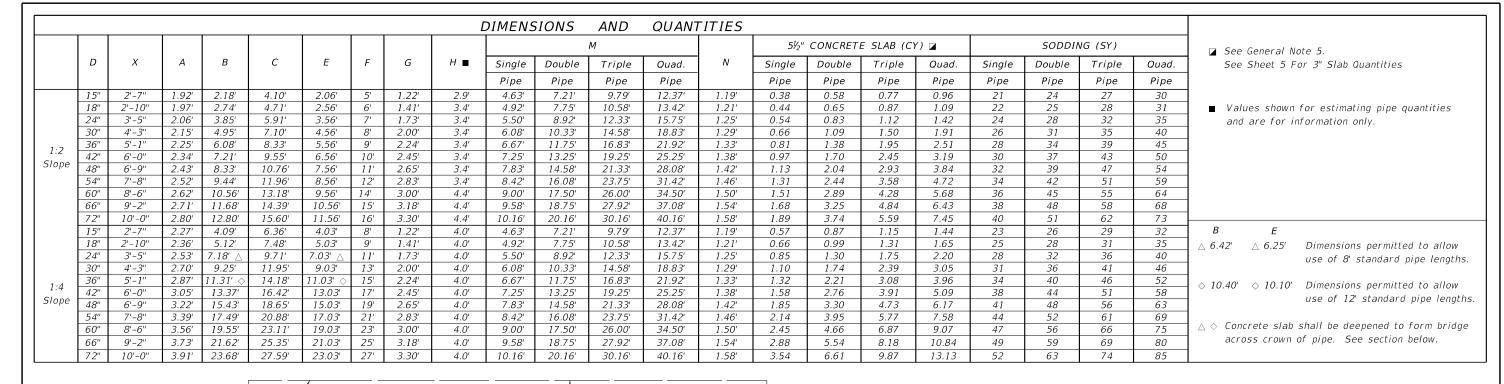


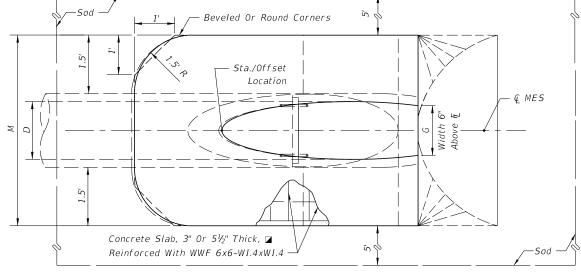
DIA.	Т	REINF. (in²/ft)	BELL Or Spigot	А	В	С	D	Ε	Р	R 1	R 2	FLAT	WEIGHT (lb)	h	TOE WALL CLASS I CONC (CY)
12"	2"	0.07	1 1/2"	4"	2'-0"	4'-0 ⁷ / ₈ ''	6'-07/8"	2'-0"	19 ¹⁵ / ₁₆ "	10½"	9"	31/2"	530	12"	.06
15"	21/4"	0.07	2"	6"	2'-3"	3'-10"	6'-1"	2'-6"	24 ⁵ ⁄ ₁₆ "	121/2"	11"	31/2"	740	12"	.07
18"	21/2"	0.07	21/2"	9"	2'-3"	3'-10"	6'-1"	3'-0"	29"	15½"	12"	4"	990	15"	.11
21"	23/4"	0.07	21/4"	9"	2'-11"	3'-2"	6'-1"	3'-6"	31%"	16½"	13"	4"	1280	15"	.12
24"	3"	0.07	21/2"	91/2"	3'-7 ¹ / ₂ "	2'-6"	6'-1½"	4'-0"	33¾ ₁₆ "	16 ¹³ / ₁₆ "	14"	41/2"	1520	18"	.17
27"	31/4"	0.148	2½"	10½"	4'-0"	2'-11/2"	6'-1½"	4'-6"	36"	18% ₁₆ "	141/2"	4½"	1930	18"	.19
30"	31/2"	0.148	3"	1'-0"	4'-6"	1'-7¾"	6'-13/4"	5'-0"	37"	18½"	15"	5"	2190	21"	.24
36"	4"	0.148	3½"	1'-3"	5'-3"	2'-10¾"	8'-1¾''	6'-0"	47 ¹³ / ₁₆ "	245/ ₁₆ "	20"	5½"	4100	21"	.29
42"	41/2"	0.148	3¾"	1'-9"	5'-3"	2'-11"	8'-2"	6'-6"	53½"	27½"	22"	5½"	5380	24"	.36
48"	5"	0.148	4½"	2'-0"	6'-0"	2'-2"	8'-2"	7'-0"	56½"	28½"	22"	5¾"	6550	24"	.39
54"	51/2"	0.174	43/4"	2'-3"	5'-5"	2'-11"	8'-4"	7'-6"	65½"	331/8"	24"	61/4"	8040	24"	.42
60"	6"	0.174	5"	2'-6"	5'-0"	3'-3"	8'-3"	8'-0"	72½"	36 ¹ 1/ ₁₆ "	24"	6¾"	8750	24"	.44
66"	6½"	0.174	5½"	2'-0"	6'-6"	1'-9"	8'-3"	8'-6"	72"	36½"	24"	71/4"	10630	24"	.47
72"	7"	0.174	6"	2'-0"	6'-6"	1'-9"	8'-3"	9'-0"	77 ¹³ / ₁₆ "	38 ¹⁵ / ₁₆ "	24"	73/4"	12520	24"	.50

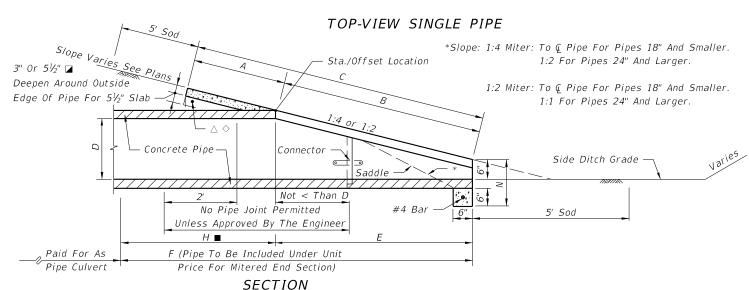
#### GENERAL NOTES

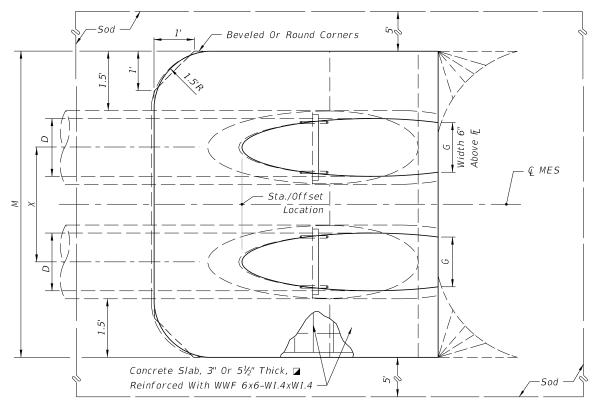
- 1. Flared end sections shall conform to the requirements of ASTM C76 with the exception that dimensions and reinforcement shall be as prescribed in the table above. Circumferential reinforcement may consist of either one cage or two cages of steel. Fiber-reinforced concrete may be substituted for conventional reinforcement in accordance with Structures Design Guidelines, Section 3.17. Compressive strength of concrete shall be 4000 psi. Shop drawings for flared end sections having fiber reinforcing or dimensions other than above must be submitted for approval to the State Drainage Engineer.
- 2. Connections between the flared end section and the pipe culvert may be any of the following types unless otherwise shown on the plans.
- a. Joints meeting the requirements of Section 449 of the Standard Specifications (O-Ring Gasket). Flared end section joint dimensions and tolerances shall be identical or compatible to those used in the pipe culvert joint. When pipe culvert and flared end section manufacturers are different, the compatibility of joint designs shall be certified to by the manufacturer of the flared end sections.
- b. Joints sealed with preformed plastic gaskets. The gaskets shall meet the requirements of Section 942-2 of the Standard Specifications and the minimum sizes for gaskets shall be as that specified for equivalent sizes of
- c. Reinforced concrete jackets, as detailed on this drawing. Cost of the reinforced concrete jacket to be included in the contract unit price for the flared end section. When non-coated corrugated metal pipe is called for in the plans, the pipe shall be bituminous coated in the jacketed area as specified on Index 430-001. Bituminous coating to be included in the contract unit price for the pipe culvert. Concrete jacket shall be as specified on Index 430-001. Cost of concrete and reinforcement shall be included in the contract unit price for the pipe culvert.
- 3. Toe walls shall be constructed when shown on the plans or at locations designated by the Engineer. Toe walls are to be cast-in-place with Class I Concrete and paid for under the contract unit price for Flared End Section (Concrete), EA. Reinforcing steel shall also be included in the cost of the Flared End Section (Concrete), EA.
- 4. On skewed pipe culverts the flared end sections shall be placed in line with the pipe culvert. Side slopes shall be warped as required to fit the flared end sections.
- 5. Flared End Section to be paid for under the contract unit price for Flared End Section (Concrete), EA. Sodding shall be in accordance with Index 524-001, and paid for under the contract unit price for Performance Turf, SY.

DESCRIPTION:









TOP-VIEW MULTIPLE PIPE

NOTE: See sheet 6 for details and notes.

# SINGLE AND MULTIPLE ROUND CONCRETE PIPE

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

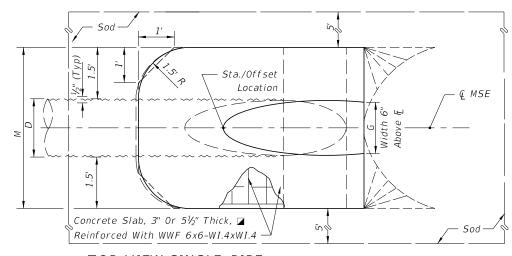
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SHEET

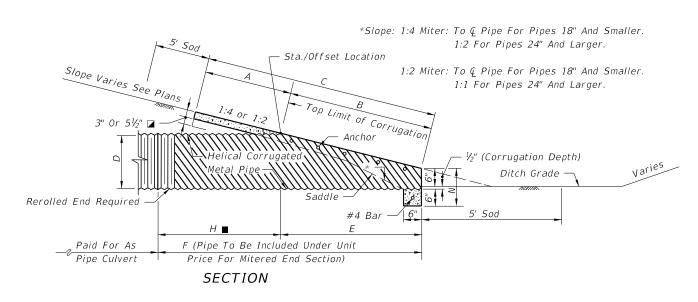
									DII	MENSIO	NS AN	ID QU	ANTITIE	S									
												М			51/2	" CONCRET	E SLAB (CY	) 🗖		SODDIN	G (SY)		]
	D	X	A	В	С	Ε	F	G	H <b>■</b>	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	N	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	
	15"	2'-7"	2.5'	1.68'	4.18'	1.5'	5.0'	1.23'	3.5'	4.33'	6.92'	9.50'	12.08'	1.04'	0.35	0.54	0.74	0.94	21	24	27	29	]
	18"	2'-10"	2.5'	2.24'	4.74'	2.0'	6.0'	1.41'	4'	4.58'	7.42'	10.25'	13.08'	1.04'	0.38	0.62	0.87	1.12	22	25	28	31	
	24"	3'-5"	2.5'	3.35'	5.85'	3.0'	7.0'	1.73'	4'	5.08'	8.50'	11.92'	15.33'	1.04'	0.47	0.76	1.05	1.34	23	27	31	35	
1.2	30"	4'-3"	2.5'	4.47'	6.97'	4.0'	8.0'	2.00'	4'	5.58'	9.83'	14.08'	18.33'	1.04'	0.57	0.96	1.37	1.77	25	30	35	39	
Slope	36"	5'-1"	2.5'	5.59'	8.09'	5.0'	9.0'	2.24'	4'	6.08'	11.17'	16.25'	21.33'	1.04'	0.67	1.19	1.72	2.26	27	33	38	44	_
Jope	42"	6'-0"	2.5'	6.71'	9.21'	6.0'	10.0'	2.45'	4'	6.58'	12.58'	18.58'	24.58'	1.04'	0.78	1.48	2.17	2.87	29	36	42	49	╛-
	48"	6'-9"	2.5'	7.83'	10.33'	7.0'	11.0'	2.65'	4'	7.08'	13.83'	20.58'	27.33'	1.04'	0.89	1.71	2.54	3.36	31	38	46	53	<b>-</b> ■
	54"	7'-8"	2.5'	8.94'	11.44'	8.0'	12.0'	2.83'	4'	7.58'	15.25'	22.92'	30.58'	1.04'	1.02	2.06	3.10	4.14	33	41	50	58	
	60"	8'-6"	2.5'	10.06'	12.56'	9.0'	13.0'	3.00'	4'	8.08'	16.58'	25.08'	33.58'	1.04'	1.14	2.38	3.63	4.89	34	44	53	63	_
	15"	2'-7"	2.5'	3.09'	5.59'	3.0'	7.0'	1.23'	4'	4.33'	6.92'	9.50'	12.08'	1.04'	0.44	0.68	0.91	1.15	22	25	28	31	
	18"	2'-10"	2.5'	4.12'	6.62'	4.0'	8.0'	1.41'	4'	4.58'	7.42'	10.25'	13.08'	1.04'	0.49	0.77	1.03	1.31	24	27	30	33	
	24"	3'-5"	2.5'	6.18'	8.68'	6.0'	10.0'	1.73'	4'	5.08'	8.50'	11.92'	15.33'	1.04'	0.65	1.09	1.38	1.77	27	30	34	38	_
1:4	30"	4'-3"	2.5'	8.25'	10.75'	8.0'	12.0'	2.00'	4'	5.58'	9.83'	14.08'	18.33'	1.04'	0.81	1.34	1.90	2.44	29	34	39	44	_
Slope	36"	5'-1"	2.5'	10.31'	12.81'	10.0'	14.0'	2.24'	4'	6.08'	11.17'	16.25'	21.33'	1.04'	0.97	1.68	2.41	3.14	32	38	44	49	_
1 3.000	42"	6'-0"	2.5'	12.37'	14.87'	12.0'	16.0'	2.45'	4'	6.58'	12.58'	18.58'	24.58'	1.04'	1.13	2.08	3.06	4.02	35	42	48	55	_
	48"	6'-9"	2.5'	14.43'	16.93'	14.0'	18.0'	2.65'	4'	7.08'	13.83'	20.58'	27.33'	1.04'	1.29	2.49	3.69	4.88	38	46	53	60	4
	54"	7'-8"	2.5'	16.49'	18.99'	16.0'	20.0'	2.83'	4'	7.58'	15.25'	22.92'	30.58'	1.04'	1.48	2.98	4.47	5.98	41	49	58	66	_
	60"	8'-6"	2.5'	18.55'	21.05'	18.0'	22.0'	3.00'	4'	8.08'	16.58'	25.08′	33.58'	1.04'	1.66	3.49	5.31	7.13	44	53	63	72	$\perp$

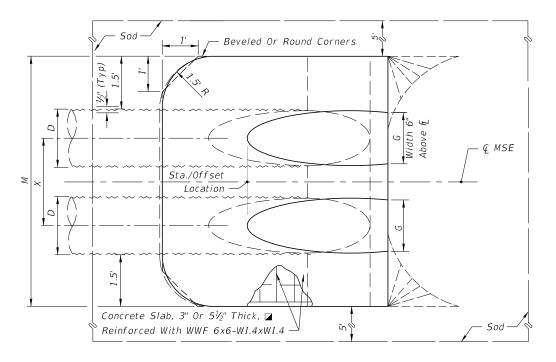
See Sheet 5 For 3" Slab Quantities

■ Values shown for estimating pipe quantities and are for information only



TOP VIEW-SINGLE PIPE





TOP VIEW-MULTIPLE PIPE

NOTE: See Sheet 6 For Details And Notes.

SINGLE AND MULTIPLE ROUND CORRUGATED METAL PIPE

REVISION 11/01/17

DESCRIPTION:

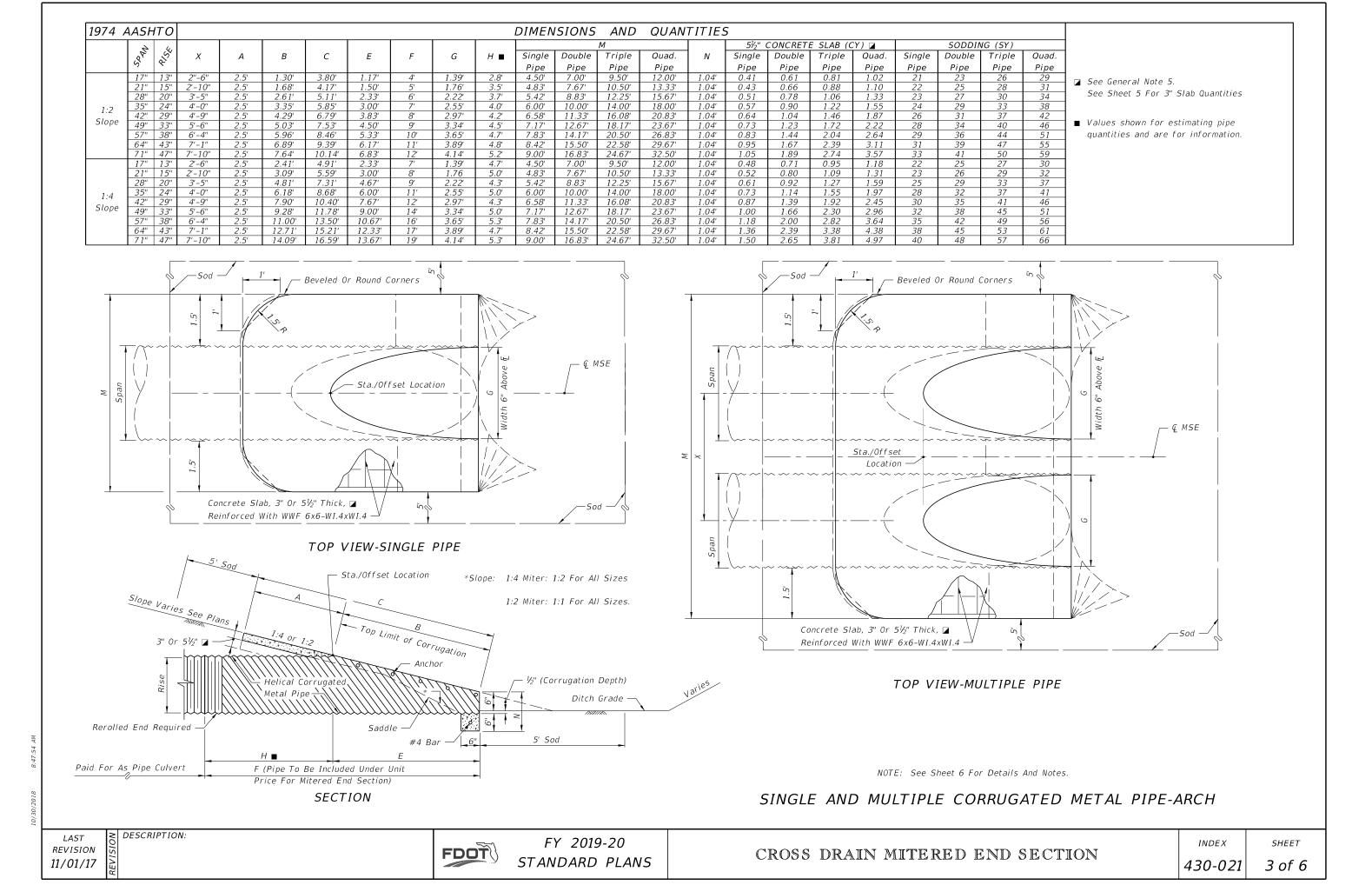
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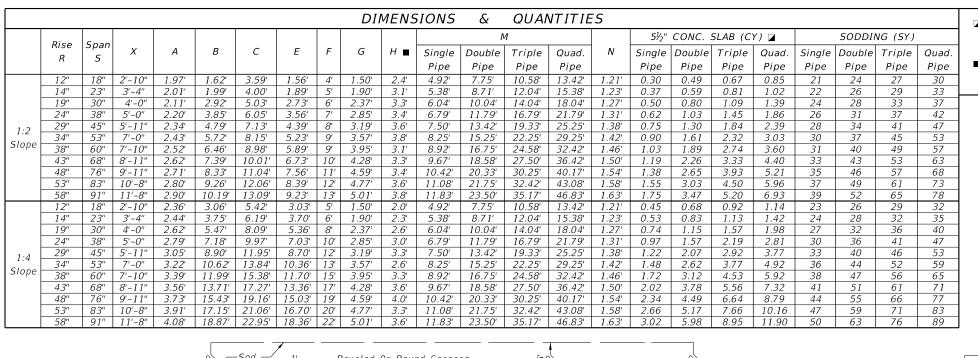
FY 2019-20 STANDARD PLANS

CROSS DRAIN MITERED END SECTION

INDEX 430-021

SHEET 2 of 6





DESCRIPTION:

REVISION

11/01/17

✓ See General Note 3.

See Sheet 5 For 3" Slab Quantities

CROSS DRAIN MITERED END SECTION

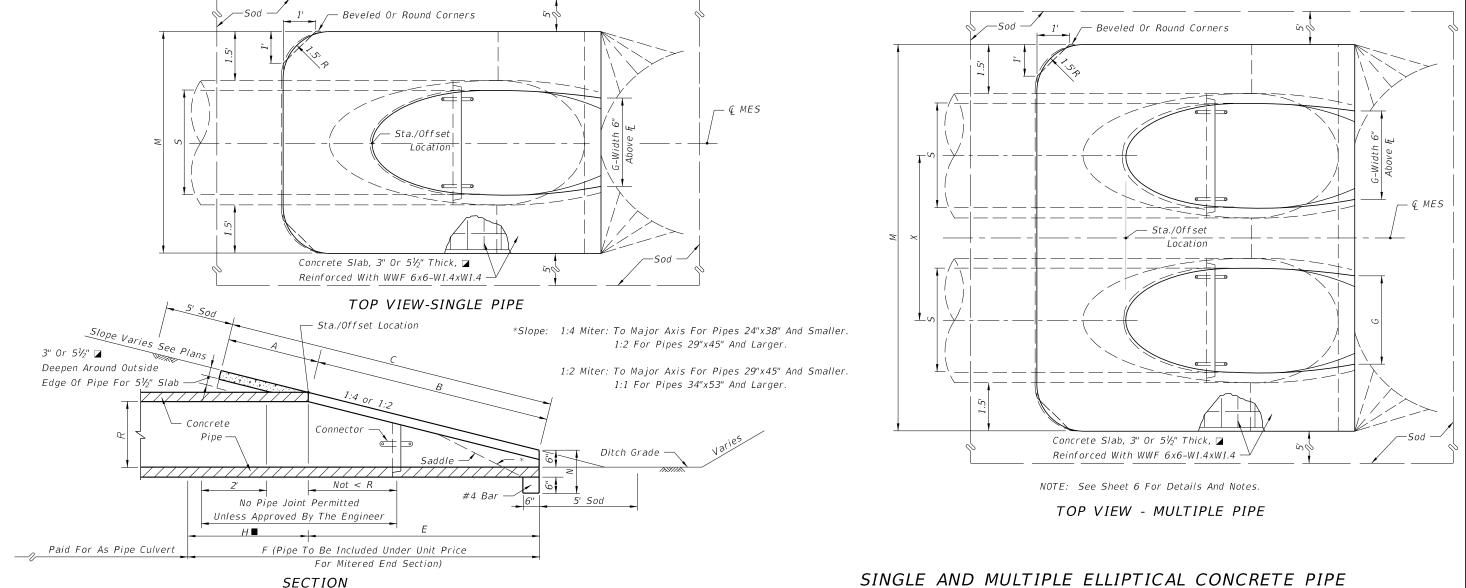
 Values shown for estimating pipe quantities and are for information only.

SHEET

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430-021



FY 2019-20

STANDARD PLANS

FDOT

# QUANTITIES FOR 3" THICK CONCRETE SLABS (CY)

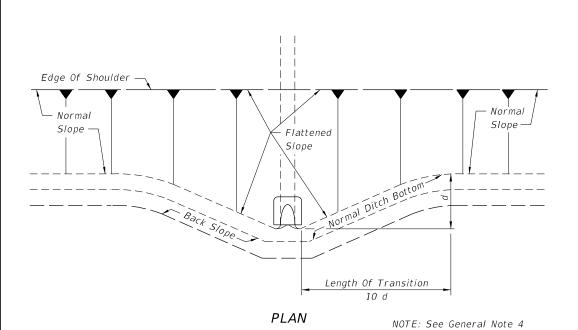
		RC	UND-C	ONCRE	ΤΕ
	D	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	15"	0.27	0.41	0.54	0.67
	18"	0.31	0.45	0.60	0.75
	24"	0.39	0.59	0.79	1.00
	30"	0.46	0.76	1.04	1.32
	36"	0.55	0.94	1.33	1.71
1:2	42"	0.66	1.15	1.66	2.15
Slope	48"	0.76	1.37	1.96	2.57
	54"	0.87	1.62	2.38	3.14
	60"	0.99	1.90	2.81	3.73
	66"	1.11	2.15	3.21	4.27
	72"	1.24	2.46	3.68	4.90
	15"	0.40	0.61	0.80	1.00
	18"	0.47	0.69	0.91	1.14
	24"	0.60	0.90	1.21	1.52
	30"	0.76	1.19	1.63	2.07
1.1	36"	0.89	1.48	2.05	2.63
1:4	42"	1.05	1.82	2.57	3.34
Slope	48"	1.21	2.15	3.07	4.00
	54"	1.39	2.55	3.72	4.88
	60"	1.59	3.02	4.44	5.86
	66"	1.91	3.66	5.40	7.15
	72"	2.12	4.18	6.24	8.30

			ROUNI	D-CMP	
	D	Single	Double	Triple	Quad.
		Pipe	Pipe	Pipe	Pipe
	15"	0.24	0.37	0.51	0.64
	18"	0.26	0.43	0.61	0.78
	24"	0.32	0.52	0.72	0.91
	30"	0.38	0.64	0.91	1.18
	36"	0.44	0.78	1.13	1.48
1:2	42"	0.51	0.96	1.41	1.87
Slope	48"	0.57	1.09	1.63	2.15
	54"	0.65	1.32	1.99	2.66
	60"	0.71	1.49	2.28	3.07
	15"	0.31	0.47	0.63	0.79
	18"	0.34	0.53	0.71	0.90
	24"	0.44	0.69	0.92	1.18
	30"	0.53	0.88	1.25	1.60
1:4	36"	0.62	1.07	1.53	2.00
	42"	0.71	1.30	1.92	2.52
Slope	48"	0.80	1.54	2.29	3.02
	54"	0.91	1.83	2.74	3.67
	60"	1.02	2.15	3.27	4.39

	u	9		CMP-	ARCH	
	Span	Rise	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	17"	13"	0.33	0.49	0.65	0.81
	21"	15"	0.33	0.50	0.67	0.83
	28"	20"	0.37	0.56	0.76	0.95
	35"	24"	0.40	0.62	0.84	1.07
	42"	29"	0.43	0.70	0.98	1.25
1:2	49"	33"	0.49	0.82	1.15	1.48
Slope	57"	38"	0.55	0.95	1.35	1.75
	64"	43"	0.62	1.10	1.57	2.05
	71"	47"	0.69	1.24	1.80	2.35
	17"	13"	0.38	0.56	0.74	0.92
	21"	15"	0.39	0.59	0.80	0.95
	28"	20"	0.43	0.64	0.88	1.10
	35"	24"	0.49	0.77	1.05	1.33
, ,	42"	29"	0.57	0.92	1.27	1.62
1:4	49"	33"	0.65	1.08	1.50	1.93
Slope	57"	38"	0.76	1.30	1.83	2.37
	64"	43"	0.87	1.55	2.18	2.83
	71"	47"	0.95	1.68	2.43	3.17

		٦	ELLI	PTICAL-	·CONCR	ETE
	Rise	Span	Single Pipe	Double	Triple	Quad.
			ripe	Pipe	Pipe	Pipe
	12"	18"	0.19	0.33	0.45	0.57
	14"	23"	0.25	0.40	0.55	0.69
	19"	30"	0.34	0.55	0.75	0.95
	24"	38"	0.43	0.71	1.00	1.28
	29"	45"	0.52	0.90	1.27	1.65
1:2	34"	53"	0.62	1.11	1.60	2.09
Slope	38"	60"	0.70	1.29	1.87	2.46
	43"	68"	0.81	1.54	2.26	2.99
	48"	76"	0.93	1.79	2.66	3.53
	53"	83"	1.04	2.04	3.03	4.02
	58"	91"	1.17	2.33	3.49	4.66
	12"	18"	0.30	0.45	0.61	0.76
	14"	23"	0.36	0.56	0.76	0.95
	19"	30"	0.51	0.79	1.08	1.36
	24"	38"	0.68	1.10	1.53	1.96
1:4	29"	45"	0.86	1.45	2.04	2.63
Slope	34"	53"	1.02	1.81	2.60	3.39
Stope	38"	60"	1.18	2.14	3.10	4.05
	43"	68"	1.38	2.58	3.79	4.99
	48"	76"	1.59	3.05	4.51	5.97
	53"	83"	1.80	3.50	5.19	6.88
	58"	91"	2.04	4.04	6.05	8.05

≥ DESCRIPTION:



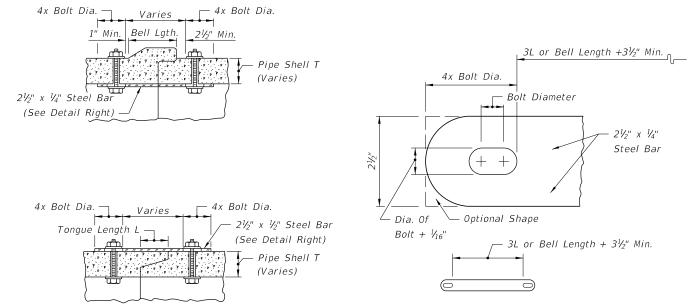
SLOPE AND DITCH TRANSITIONS

### GENERAL NOTES

- 1. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of cross drain pipe; corrugated steel pipe mitered end sections may be used with any type of cross drain pipe of cross drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of cross drain pipe except steel pipe. When bituminous coated metal pipe is specified for cross drain pipe, construct the mitered end sections with like pipe or concrete pipe. When the mitered end section pipe is dissimilar to the cross drain pipe, construct a concrete jacket in accordance with Index 430-001.
- 2. Use either corrugated metal or concrete mitered end sections for corrugated polyethylene pipe (HDPE), polyvinyl-chloride pipe (PVC) and polypropylene pipe (PPP). When used in conjunction with corrugated mitered end sections, make connection using either a formed metal band specifically designated to join HDPE or PVC pipe, with metal pipe or other coupler approved by the State Drainage Engineer. When used in conjunction with a concrete mitered end sections, concrete jacket constructed in accordance with Index 430-001.
- 3. Class NS concrete cast-in-place reinforced slabs are required for all sizes of cross drain pipes. Unless 3" thickness called for in plans, construct slabs at 5½" thick.
- 4. Select lengths of concrete pipe that avoid excessive connections in the assembly of the mitered end section.
- 5. Repair corrugated metal pipe galvanizing that is damaged during beveling and perforating.
- 6. Prior to placing concrete slab apply a bituminous coating to any portion of corrugated metal pipe in direct contact with concrete. Extend the coating 12" beyond the concrete slab.
- 7. When existing multiple cross drain pipes are spaced other than the dimensions shown in this Index, have nonparallel axes, or non-uniform sections, either construct the mitered end sections separately as single pipe or collectively as multiple pipe end sections as directed by the Engineer.

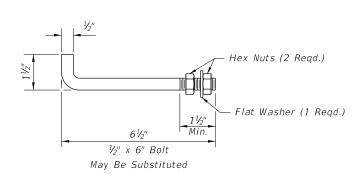
### **DESIGN NOTES**

- 1. Mitered end sections for pipe sizes 15", 18" and 24" round or equivalent pipe arch or elliptical pipe are permitted within the clear zone. When the slope intersection permits, the mitered end section may be located with the culvert opening as close as 8' beyond the outside edge of the shoulder.
- 2. Include slope and ditch transitions when the normal roadway slope must be flattened to place end section outside clear zone. See Slope and Ditch Transitions detail.



All bars, bolts, nuts and washers are to be galvanized steel. Bolt diameters shall be  $\frac{3}{8}$ " for 15" to 36" pipe and  $\frac{5}{8}$ " for 42" to 72" pipe. Two connectors required per joint, located 60° right and left of bottom center of pipe. Bolt holes in pipe shell are to be drilled.

## CONCRETE PIPE CONNECTOR



Anchors required for CMP only.

Anchor, washer and nuts to be galvanized steel.

Bend anchor where required to center in concrete slab. Damaged surfaces to be repaired after bending. Anchors are to be spaced a distance equal to four (4) corrugations. Place the anchors in the outside crest of corrugation.

Flat washers to be placed on inside wall of pipe.

Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

# ANCHOR DETAIL SPECIAL DETAILS AND NOTES

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS SPECIAL DETAILS AND NOTES

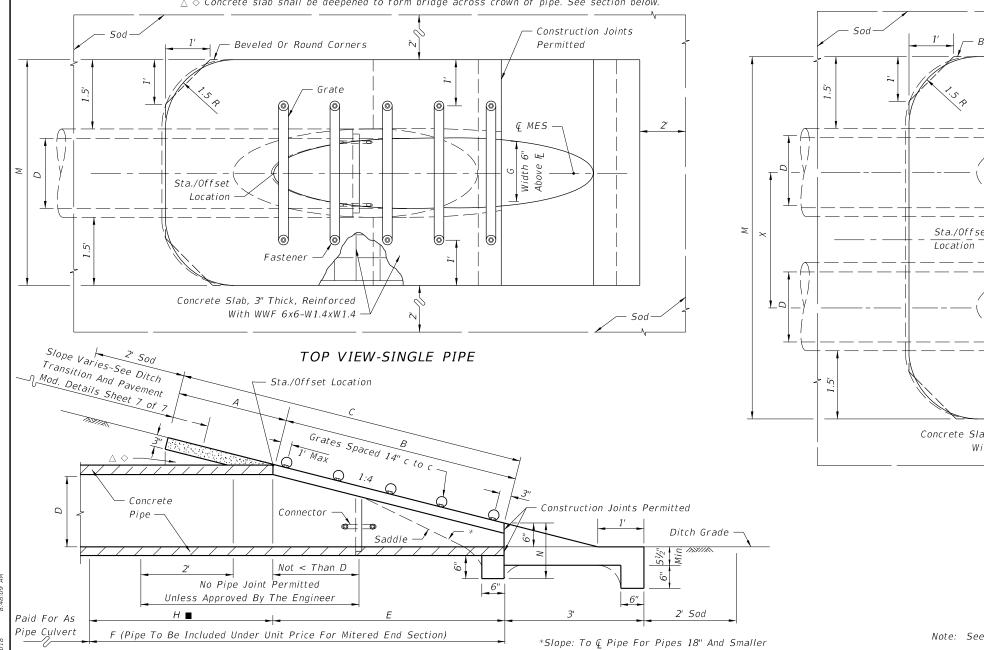
INDEX

430-021

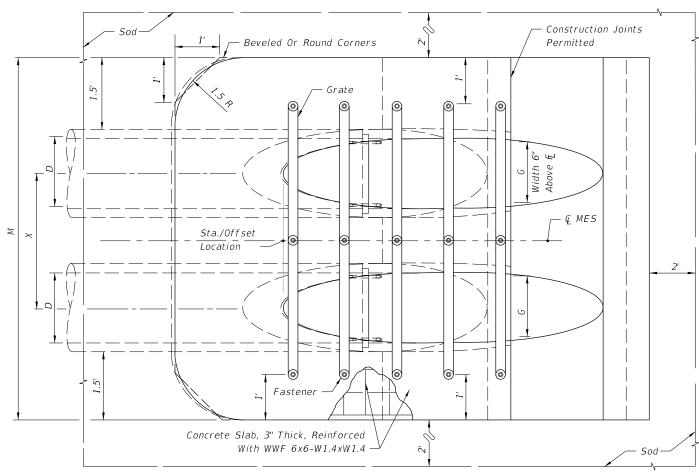
SHEET

										DI	MENSIO	NS &	QUAN	TITIES									
											М			GRATE	SIZES		CONCRET	E (CY)			SODDING	G (SY)	
D	X	A	В	С	E	F	G	H <b>■</b>	Single Pipe	Double Pipe	Triple Pipe	Quad Pipe	N	Standard Weight Pipe	Extra Strong Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad Pipe
15"	2'-7"	2.27'	4.09'	6.36'	4.03'	8'	1.22'	4.0'	4.63'	7.21'	9.79'	12.37'	1.19'			0.76	1.16	1.54	1.94	8	10	11	12
18"	2'-10"	2.36'	5.12'	7.48'	5.03'	9'	1.41'	4.0'	4.92'	7.75'	10.58'	13.42'	1.21'			0.85	1.28	1.71	2.17	9	10	12	13
24"	3'-5"	2.53'	7.18' △	9.71'	7.03' A	11'	1.73'	4.0'	5.50'	8.92'	12.33'	15.75'	1.25'			1.02	1.58	2.15	2.75	10	12	13	15
30"	4'-3"	2.70'	9.25'	11.95'	9.03'	13'	2.00'	4.0'	6.08'	10.33'	14.58'	18.83'	1.29'	21/2"	3"	1.23	1.98	2.74	3.50	12	14	15	17
36"	5'-1"	2.87'	11.31' ♦	14.18'	11.03' ♦	15'	2.24'	4.0'	6.67'	11.75'	16.83'	21.92'	1.33'	21/2"	3"	1.40	2.38	3.33	4.24	13	15	17	20
42"	6'-0"	3.05'	13.37'	16.42'	13.03'	17'	2.45'	4.0'	7.25'	13.25'	19.25'	25.25'	1.38'	21/2"	31/2"	1.60	2.83	4.04	5.26	14	17	19	22
48"	6'-9"	3.22'	15.43'	18.65'	15.03'	19'	2.65'	4.0'	7.83'	14.58'	21.33'	28.08'	1.42'	21/2"	31/2"	1.81	3.26	4.70	6.14	15	18	21	24
54"	7'-8"	3.39'	17.49'	20.88'	17.03'	21'	2.83'	4.0'	8.42'	16.08'	23.75'	31.42'	1.46'	3"	4"	2.03	3.78	5.54	7.28	17	20	23	27
60"	8'-6"	3.56'	19.55'	23.11'	19.03'	23'	3.00'	4.0'	9.00'	17.50'	26.00'	34.50'	1.50'	3"	4"	2.28	4.36	6.43	8.50	18	22	25	29

- $\triangle$  6.42'  $\triangle$  6.25' Dimensions permitted to allow use of 8' standard pipe lengths.
- $\diamond$  10.40'  $\diamond$ 10.10' Dimensions permitted to allow use of 12' standard pipe lengths.
- $\triangle \diamondsuit$  Concrete slab shall be deepened to form bridge across crown of pipe. See section below.



■ Values shown for estimating pipe quantities and are for information only.



TOP VIEW-MULTIPLE PIPE

Note: See Sheets 6 and 7 for details and general notes.

# SINGLE AND MULTIPLE ROUND CONCRETE PIPE

REVISION 11/01/17

DESCRIPTION:

FDOT

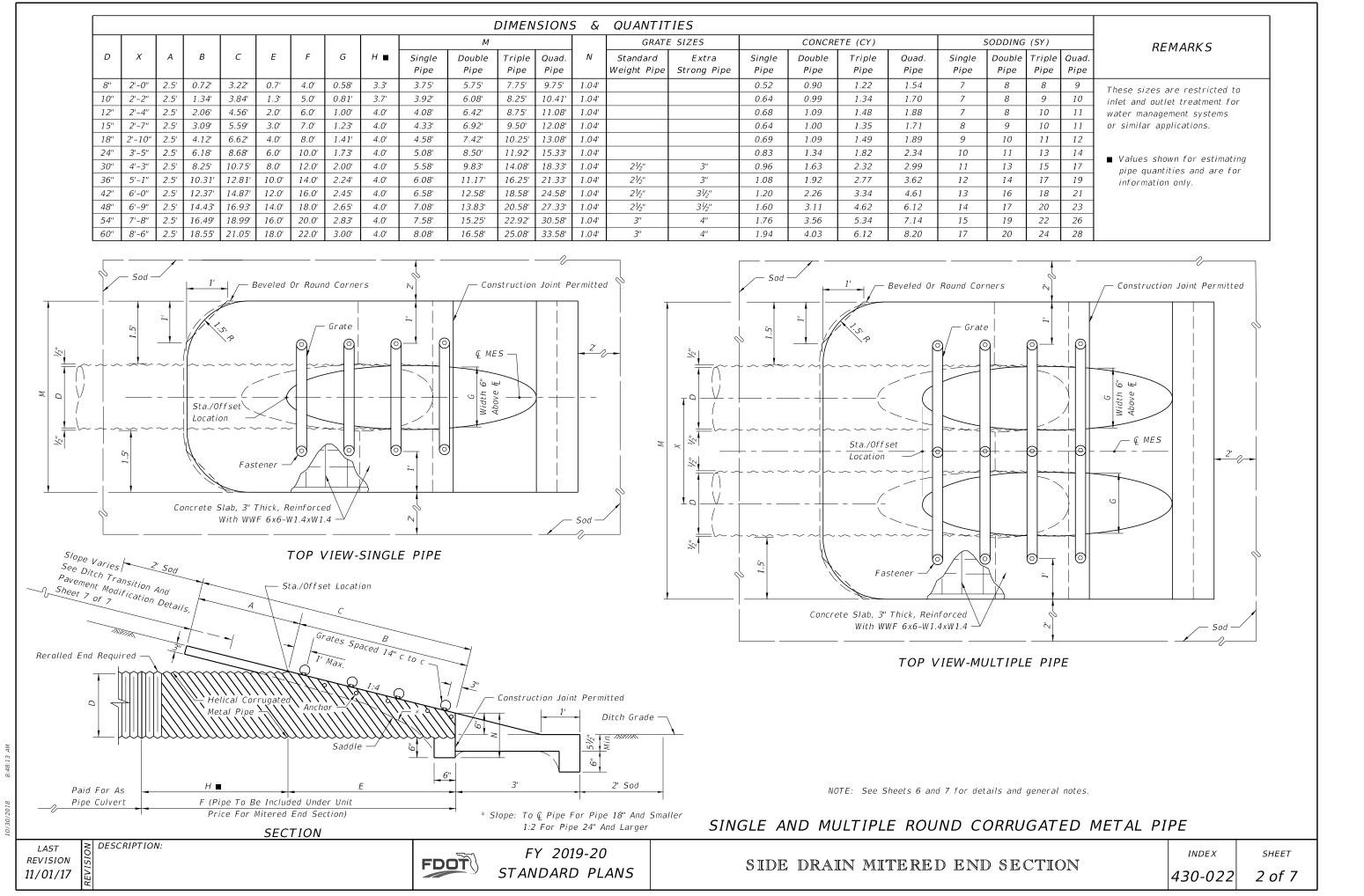
**SECTION** 

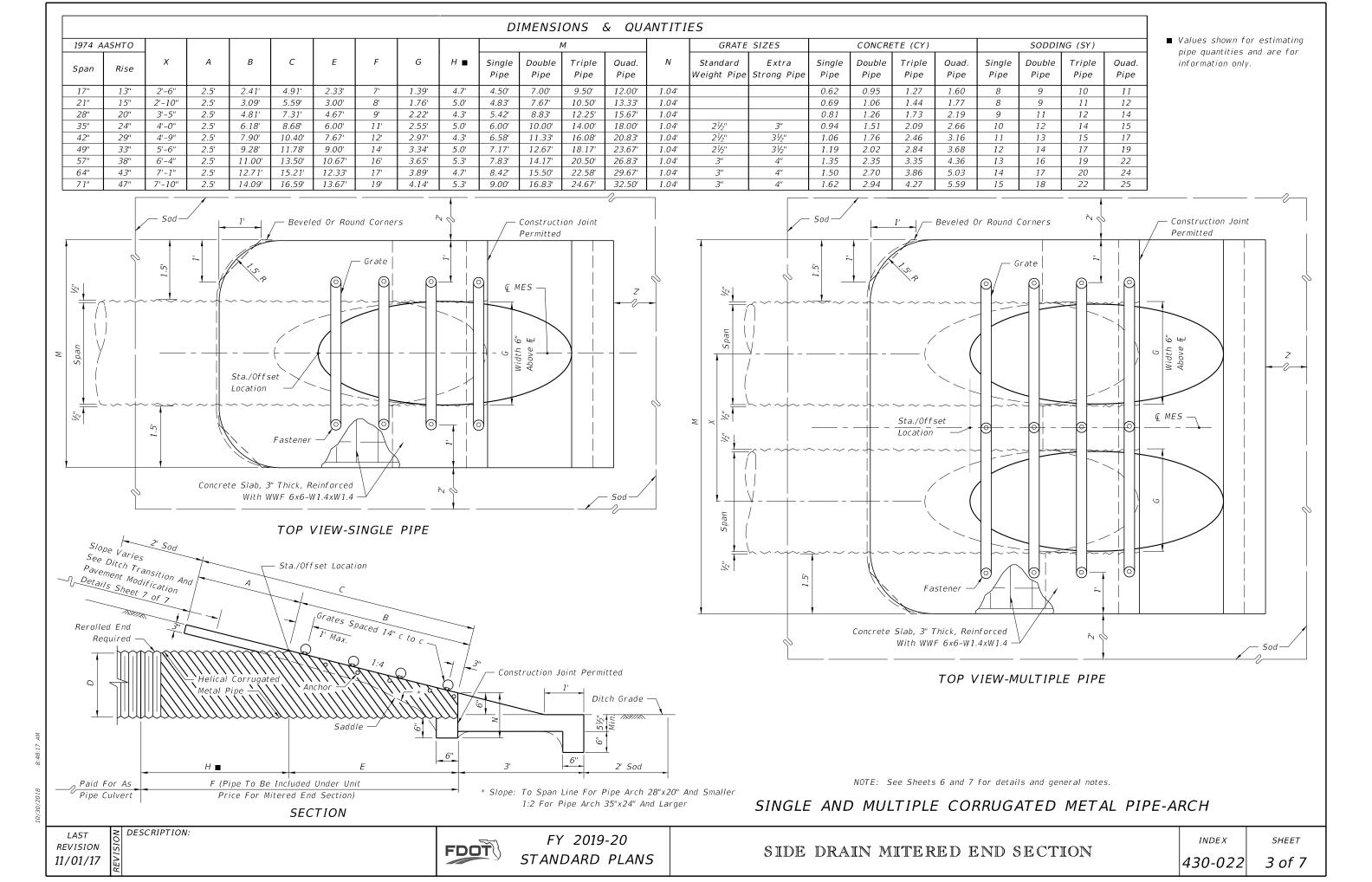
FY 2019-20 STANDARD PLANS

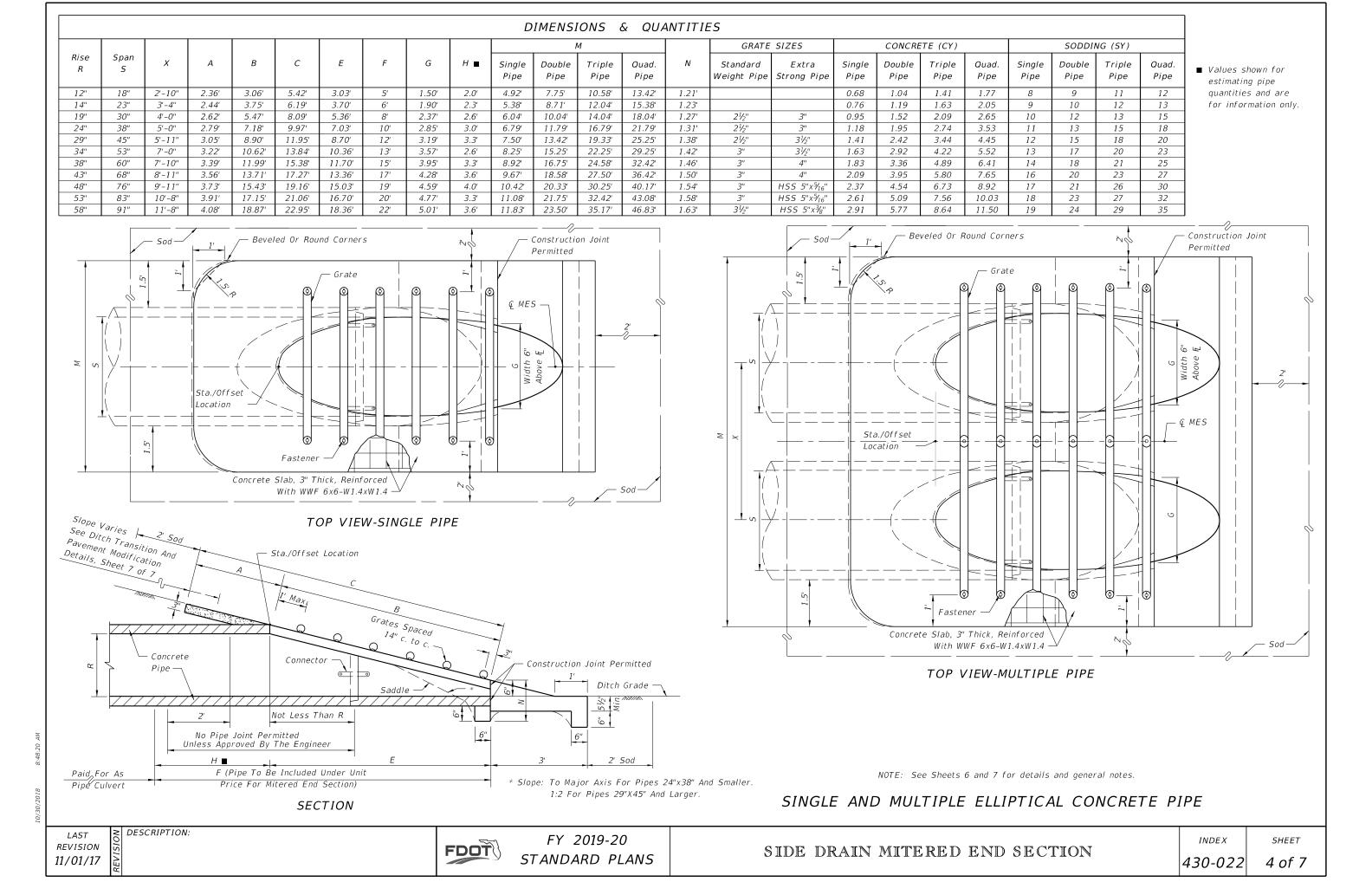
1:2 For Pipes 24" And Larger.

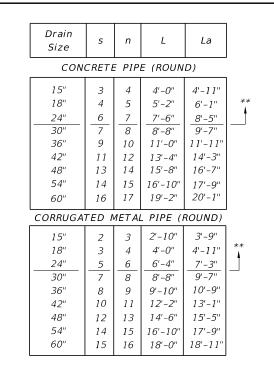
INDEX

SHEET 1 of 7









Drain Size	s	n	L	La	
ELLIPT	rical	CON	CRETE P	IPE	
12"x18"	2	3	2'-10"	3'-9"	**
14"x23"	3	4	4'-0'	4'-11"	1
19"x30"	4	-5	5'-2"	6'-1"	_
24"x38"	5	6	6'-4"	7'-3"	
29"x45"	7	8	8'-8"	9'-7"	
34"x53"	8	9	9'-10"	0'-9"	
38"x60"	10	11	12'-2"	13'-1"	
43"x68"	11	12	13'-4"	14'-3"	
48"x76"	13	14	15'-8"	16'-7"	
53"x83"	14	15	16'-10"	17'-9"	
58"x91"	15	16	18'-0"	18'-11"	
CORRUGA	ATED	META	AL PIPE	(ARCH)	***
17"x13"	1	2	1'-8"	2'-7"	**
21"x15"	2	3	2'-10"	3'-9"	Ĭ
28"x20"	4	5	5'-2"	6'-1"	
35"x24"	5	6	6'-4"	7'-3"	
42"x29"	6	7	7'-6"	8'-5"	
1	I	1 -	1	I .	I

11'-0"

12'-2"

14'-6"

11'-11"

13'-1"

15'-5"

10

11

13

10

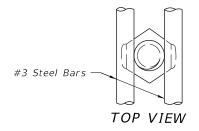
12

Note: 5%" x 3" bolts are standard for all grate fasteners, except when the contractor elects to use the slotted upper holes for the intermediate fasteners on multiple drain pipes, which will require the following bolt lengths:

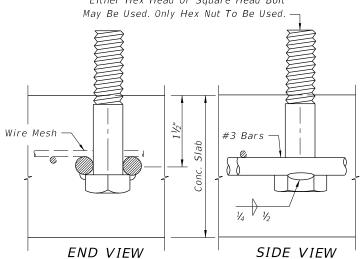
Grate Size (Std. & X-Stg.)	Bolt Length
21/2"	5½"
3"	6"
31/2"	6½"
4"	7"

** To be used only when grates are called for in the plans.

*** 1974 AASHTO Pipe Arch Sizes.

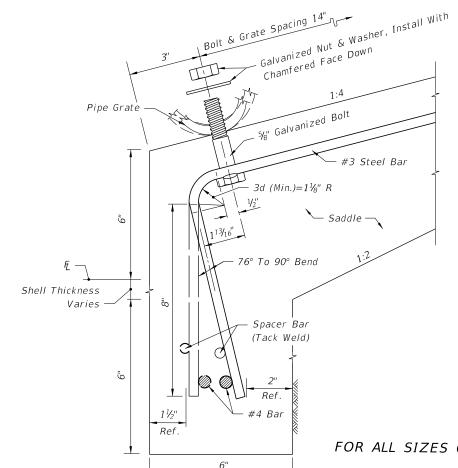


%" Galvanized Bolt Hex Head Bolt Shown; Either Hex Head Or Square Head Bolt May Be Used. Only Hex Nut To Be Used. -





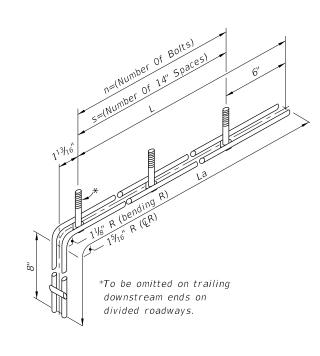
The specified weld shall be made when the fabricated unit is subject to hazardous hauls and repeated handling. Tack welds are permitted for local or job site fabrication. Galvanizing over welded surface not required.



49"x33"

57"x38"

64"x43" 71"x47"



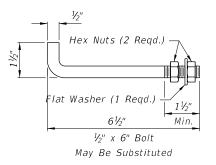
FOR ALL SIZES OF SINGLE AND MULTIPLE DRAIN PIPE FASTENER UNIT

DETAILS FOR CONCRETE & CORRUGATED METAL PIPE

LAST REVISION 11/01/17

FDOT

FY 2019-20 STANDARD PLANS



Notes:

Anchors required for CMP only.

Anchor, washer and nuts to be galvanized steel.

Bend anchor where required to center in concrete slab.

Damaged surfaces to be repaired after bending.

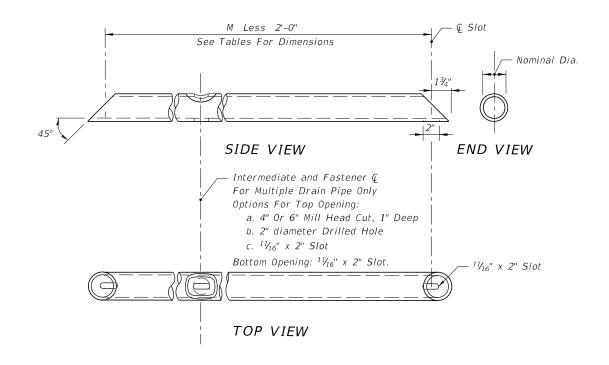
Anchors are to be spaced a distance equal to four (4) corrugations.

Place the anchors in the outside crest of corrugation.

Flat washer to be placed on inside wall of pipe.

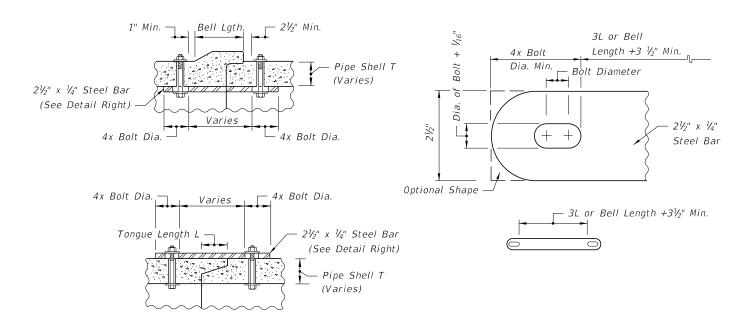
Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

## ANCHOR DETAIL



# FOR SINGLE & MULTIPLE DRAIN PIPE GRATE DETAIL

See General Notes, Sheet 7.



All bars, bolts, nuts and washers are to be galvanized steel. Bolt diameters shall be  $\frac{3}{8}$ " for 15" to 36" pipe and  $\frac{5}{8}$ " for 42" to 60" pipe. Two connectors required per joint, located 60° right and left of bottom center of pipe. Bolt holes in pipe shell are to be drilled.

CONCRETE PIPE CONNECTOR DETAIL

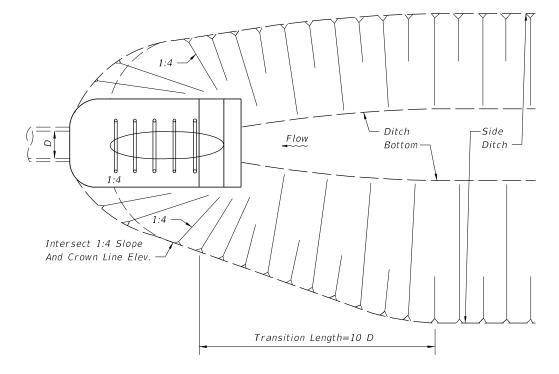
## DETAILS FOR CONCRETE & CORRUGATED METAL PIPE

**REVISION** 11/01/17

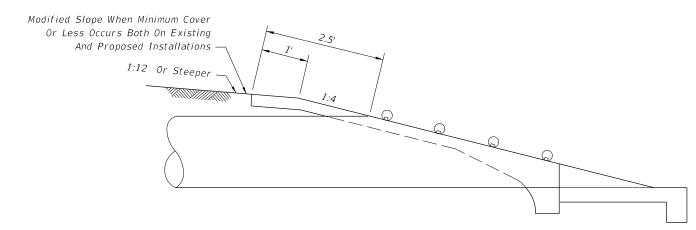
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS



## PLAN DITCH TRANSITION



## PERMISSIBLE PAVEMENT MODIFICATION

#### GENERAL NOTES

- 1. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of side drain pipe; corrugated steel pipe mitered end sections may be used with any type of side drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of side drain pipe except steel pipe. When bituminous coated metal pipe is specified for side drain pipe, construct the mitered end sections with like pipe or concrete pipe. When the mitered end section pipe is dissimilar to the side drain pipe, construct a concrete jacket in accordance with Index 430-001.
- 2. Use either corrugated metal or concrete mitered end sections for corrugated polyethylene pipe (HDPE), polyvinyl-chloride pipe (PVC) and polypropylene pipe (PPP). When used in conjunction with corrugated mitered end sections, make connection using either a formed metal band specifically designated to join HDPE or PVC pipe, with metal pipe or other coupler approved by the State Drainage Engineer. When used in conjunction with a concrete mitered end sections, concrete jacket constructed in accordance with Index 430-001.
- 3. Select lengths of concrete pipe that avoid excessive connections in the assembly of the mitered end section.
- 4. Repair corrugated metal pipe galvanizing that is damaged during beveling and perforating.
- 5. Prior to placing concrete slab apply a bituminous coating to any portion of corrugated metal pipe in direct contact with concrete. Extend the coating 12" beyond the concrete slab.
- 6. When existing multiple side drain pipes are spaced other than the dimensions shown in this Index, have nonparallel axes, or non-uniform sections, either construct the mitered end sections separately as single pipe or collectively as multiple pipe end sections as directed by the Engineer.
- 7. Class NS concrete cast-in-place reinforced slabs are required for all sizes of side drain pipes.
- 8. Install grates on all round pipes 30" or greater, pipe-arches 35"x24" or greater, and elliptical pipe 19"x30" or greater, unless excluded in the Plans. Install grates on smaller size pipes only when called for in the Plans. Omit the lower grate on the downstream end of mitered end sections along divided highways.
- 9. Use Schedule 80 pipe for the lower grate on all traffic approach ends and Schedule 40 pipe for all remaining grates. Fabricate the grates from ASTM A53, Grade B, black steel pipe and hot dip galvanize after fabrication in accordance with ASTM A123 for all corrosive environments.

### DESIGN NOTES

- 1. Do not use grates until the debris transport potential has been evaluated by the drainage engineer and appropriate adjustments made. Ditch grades in excess of 3% or pipe with less than 1.5' of cover and grades in excess of 1% will require such an evaluation (General Note 10).
- 2. The design engineer must determine and designate in the plans which alternate types of mitered end section will not be permitted. Restrict use based on corrosive or structural requirements.
- 3. Contact the District Drainage Engineer for possible alternate treatment of side drain mitered end sections where a minimum spacing of 30' will not result between the toe points of the mitered end sections.
- 4. Provide ditch transitions on all grades in excess of 3%.

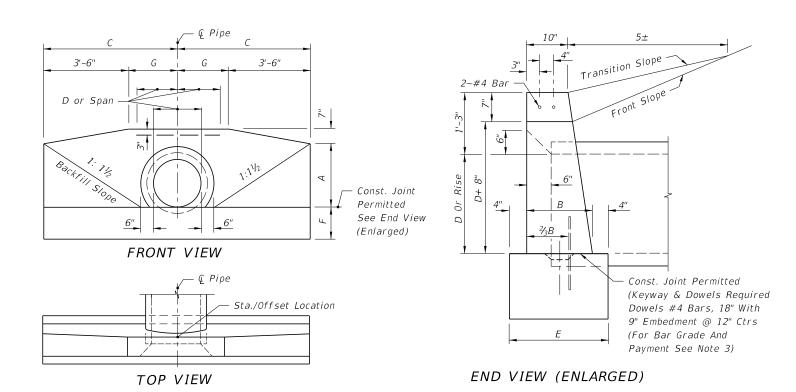
NOTES & INFORMATION

**REVISION** 11/01/17

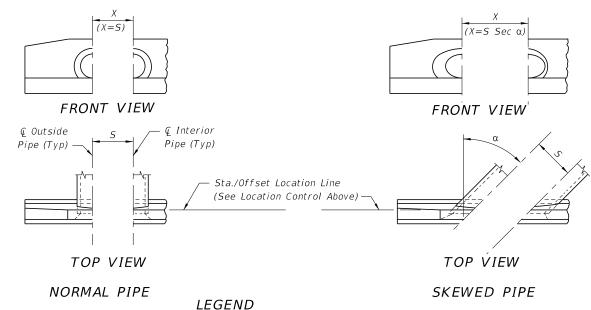
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

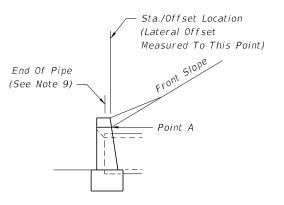


## ENDWALL DIMENSIONS (EXCLUSIVE OF MULTIPLE PIPE SPACING)



- α Pipe Skew
- S Center To Center Pipe Spacing
- X Centerline To Centerline Dimension At Face Of Headwall

PIPE AND SPACING FOR MULTIPLE PIPE ENDWALL POSITIONS FOR SINGLE AND MULTIPLE



### END VIEW

- 1. Position is set by the intersection of the front slope and Point A where this intersection falls outside the clear zone.
- 2. Where the front slope and Point A intersects inside the clear zone, the endwall is positioned so the Station/Offset Location is at the clear zone limit. The front slope is transitioned to the endwall as shown in Index 430-001.

## STANDARD LOCATION CONTROL

## GENERAL NOTES

- 1. Endwall dimensions, locations and positions are for round and elliptical concrete pipe and for round and pipe-arch corrugated metal pipe. Round concrete pipe shown.
- 2. Front slope and ditch transitions shall be in accordance with
- 3. Endwalls may be cast in place or precast concrete. Reinforcing steel shall be Grades 40 or 60. Additional reinforcement necessary for handling precast units shall be determined by the Contractor or the supplier. Cost of reinforcement shall be included in the contract unit price for Concrete, (Endwalls).
- 4. All exposed corners and edges of concrete are to be chamfered 3/4".
- 5. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 6. On outfall ditches with side slopes flatter than 1:1½ provide 20' transitions from the endwall to the flatter side slopes. right of way permitting.
- 7. For sodding around endwalls see Index 524-001.
- 8. Payment for concrete quantities for endwalls skewed to the pipe shall be made on the following basis:

Endwall Skew to Pipe Use Tabulated Value 0° to 5° 0°  $6^{\circ}$ 15° 16° 30°  $45^{\circ}$ 31° or over

- 9. Pipe length plan quantities shall be based on the pipe end locations shown in the standard location control end view, or lengths based on special endwall locations called for in the plans.
- 10. Payment for pipe in pipe culverts shall be based on plan quantities, adjusted for endwall locations subsequently established by the Engineer.
- 11. Endwalls to be paid for under the contract unit price for Class I Concrete (Endwalls), CY.

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

INDEX

SHEET

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# DATA AND ESTIMATED QUANTITIES FOR ONE ENDWALL

## ROUND CONCRETE AND CORRUGATED METAL PIPE

			pening	A																								Clas.	s I C	oncret	e (CY)												
		U	pening SF)							ı	Dimensi	ions														Numbe	er And	Туре	Of Pip	oe And	Skew	Angle	Of Pip	e									
	7		(5.	,						-	,,,,,c,,,o,						Si	ngle				Do	uble							Tri	ple							Quad	lruple				D
		Nur	nber O	f Pipes	5	Λ	R		F	F	G	5			X		Conc	Metal		Cond	rete			М	etal			Conc	rete			Ме	tal			Conci	rete		1	Mi	etal		
		1	2	3	4	_ ^	Ь	Ü	_	,			0°	15°	30°	45°	0°	0°	0°	15°	30°	45°	0°	15°	30°	$45^{\circ}$	0°	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°	
1	5" 1	23	2.46 .	3.69	4.92	1'-11"	1'-2"	4'-0"	1'-10"	1'-2"	0'-6"	2'-7"	2'-7"	2'-8"	3'-0"	3'-8"	1.23	1.24	1.59	1.60	1.65	1.74	1.62	1.63	1.68	1.78	1.94	1.96	2.05	2.23	1.99	2.02	2.11	2.30	2.30	2.34	2.47	2.74	2.37	2.41	2.75	2.84	15"
1	3" 1	.77	3.54	5.31	7.08	2'-2"	1'-3"	4'-6"	1'-11"	1'-3"	1'-0"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.56	1.59	1.99	2.01	2.06	2.17	2.04	2.06	2.11	2.23	2.43	2.46	2.56	2.79	2.51	2.54	2.65	2.89	2.86	2.91	3.06	3.40	2.96	3.01	3.17	3.53	18"
2	1" 2	.41	4.82	7.23	9.64	2'-5"	1'-4"	5'-0"	2'-0"	1'-4"	1'-6"	3'-2"	3'-2"	3'-3"	3'-8"	4'-6"	1.97																						i				21"
2	4" 3	14	6.28	9.42 1	2.56	2'-8"	1'-4"	5'-6"	2'-0"	1'-4"	2'-0"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	2.24	2.29	2.82	2.84	2.91	3.06	2.91	2.93	3.01	3.17	3.39	3.43	3.57	3.87	3.52	3.56	3.71	4.03	3.97	4.03	4.24	4.69	4.14	4.20	4.43	4.91	24"
2	7" 3	98	7.96 1	1.94 1	5.92	2'-11"	1'-5"	6'-0"	2'-1"	1'-5"	2'-6"	3'-10"	3'-10"	4'-0"	4'-5"	5'-5"	2.73																										27"
3	)" <i>4</i>	.91	9.82 1	4.73 1	9.64	3'-2"	1'-6"	6'-6"	2'-2"	1'-6"	3'-0"	4'-3"	4'-3"	4'-5"	4'-11"	6'-0"	3.26	3.34	4.13	4.16	4.26	4.49	4.28	4.31	4.43	4.67	4.98	5.04	5.25	5.69	5.20	5.27	5.49	5.97	5.84	5.93	6.24	6.91	6.13	6.23	6.56	7.29	30"
3	5" <i>7</i>	.07	14.14 2	21.21 2	8.28	3'-8"	1'-8"	7'-6"	2'-4"	1'-8"	4'-0"	5'-1"	5'-1"	5'-3"	5'-10"										6.15																		36"
4	2" 9	62	19.24 2	8.86	8.48	4'-2"	1'-10"	8'-6"	2'-6"	2'-0"	5'-0"	6'-0"	6'-0"	6'-3"	6'-11"	8'-6"	6.33	6.49	8.11	8.17	8.39	8.85	8.43	8.50	8.73	9.23	9.90	10.02	10.45	11.38	10.38	10.52	10.98	11.99	11.68	11.87	12.51	13.89	12.32	12.52	13.22	14.73	42"
4	3" 12	2.57 2	25.14 3	37.71 5	0.28	4'-8"	2'-1"	9'-6"	2'-9"	2'-0"	6'-0"	6'-9"	6'-9"	7'-0"	7'-10"										1 11.23																		48"
5	4" 15	.90	31.80 4	7.70 6	3.60	5'-2"	2'-6"	10'-6"	3'-2"	2'-3"	7'-0"	7'-8"	7'-8"	7'-11"	8'-10"	10'-10"	11.71	11.77	15.23	15.35	15.78	16.69	15.35	15.48	3 15.90	16.83	18.77	19.02	19.86	21.69	18.93	19.18	20.04	21.89	22.29	22.66	23.93	26.67	22.51	22.89	24.17	26.96	54"
																																							i				

													CORF	RUGAT	red i	1ET AL	PIPE	ARCI	4													
		(		ng Are	a					E	Dimension	S									Cla	ss I (	Concret	e (CY	)							Approx.
Span	Rise		(.	SF)																Numbe	r Of P	Pipe Ar	nd Skei	w Angl	e Of I	Pipe				Span	Rise	Equiv. Round
		Nu	mber	Of Pi	oes	_	В		_	_	-	_		,	Χ		Single		Dou	uble			Tri	ple			Quad	lruple				Pipe
		1	2	3	4	A	В	C	E	Г	G	5	0°	15°	30°	45°	0°	0°	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°			
17"	13"	1.1	2.2	3.3	4.4	1'-9"	1'-2"	3'-10"	1'-10"	1'-2"	0'-4"	2'-6"	2'-6"	2'-7"	2'-11"	3'-6"	1.16	1.47	1.48	1.52	1.60	1.78	1.80	1.88	2.04	2.09	2.12	2.23	2.48	17"	13"	15"
21"	15"	1.6	3.2	4.8	6.4	1'-11"	1'-2"	4'-3"	1'-10"	1'-2"	0'-9"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.33	1.69	1.70	1.75	1.84	2.04	2.06	2.15	2.33	2.40	2.44	2.57	2.84	21"	15"	18"
28"	20"	2.8	5.6	8.4	11.2	2'-4"	1'-3"	5'-2"	1'-11"	1'-3"	1'-8"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	1.78	2.31	2.33	2.39	2.53	2.83	2.87	2.99	3.26	3.36	3.42	3.60	4.01	28"	20"	24"
35"	24"	4.3	8.6	12.9	17.2	2'-8"	1'-4"	5'-11½"	2'-0"	1'-4"	2'-5½"	4'-0"	4'-0"	4'-2"	4'-7"	5'-8"	2.34	3.03	3.05	3.14	3.32	3.72	3.77	3.93	4.29	4.40	4.47	4.72	5.25	35"	24"	30"
42"	29"	5.9	11.8	17.7	23.6	3'-1"	1'-5"	6'-10½"	2'-1"	1'-5"	3'-41/2"	4'-9"	4'-9"	4'-11"	5'-6"	6'-9"	3.13	4.06	4.09	4.20	4.45	4.99	5.06	5.28	5.76	5.93	6.03	6.36	7.09	42"	29"	36"
49"	33"	8.4	16.8	25.2	33.6	3'-5"	1'-6"	7'-8"	2'-2"	1'-6"	4'-2"	5'-6"	5'-6"	5'-8"	6'-4"	7'-9"	3.83	5.00	5.04	5.18	5.48	6.16	6.24	6.52	7.12	7.32	7.44	7.86	8.76	49"	<i>33</i> "	42"
57"	38"	10.6	21.2	31.8	42.4	3'-10"	1'-7"	8'-7 ¹ / ₂ "	2'-3"	1'-7"	5'-1½"	6'-4"	6'-4"	6'-7"	7'-4"	8'-11"	4.87	6.31	6.36	6.53	6.91	7.74	7.84	8.18	8.93	9.18	9.33	9.85	10.96	57"	38"	48"
64"	43"	13.2	26.4	39.6	52.8	4'-3"	1'-8"	9'-61/2"	2'-4"	1'-8"	6'-0 ¹ / ₂ "	7'-1"	7'-1"	7'-4"	8'-2"	10'-0"	5.88	7.64	7.70	7.91	8.37	9.40	9.52	9.94	10.86	11.15	11.33	11.97	13.33	64"	43"	54"
71"	47"	16.9	33.8	50.7	67.6	4'-7"	1'-10"	10'-4"	2'-6"	2'-0"	6'-10"	7'-10"	7'-10"	8'-1"	9'-1"	11'-1"	7.80	10.15	10.23	10.51	11.12	12.49	12.65	13.22	14.43	14.85	15.10	15.94	17.77	71"	47"	60"

Note: Use the guidelines of General Note 8 for selecting tabular quantities.

													CC	NCRE	TE E	LLIPT.	ICAL P	IPE														
		(	Openir	ng Are SF)	a					Di	imensions	5									Ci	ass I (	Concre	te (CY)								Approx.
Rise	Span		( 2	) <i>(</i>																Numb	ber Of	Pipe A	nd Ske	w Angle	e Of Pi	pe				Rise	Span	Equiv.
		Nu	mber	Of Pi	pes	4		-	_	_	G	_		,	Υ		Single		Do	uble			Tr	iple			Quadr	uple				Round Pipe
		1	2	3	4	] ^	Б			<i>r</i>	G	3	0°	15°	30°	45°	<i>0°</i>	0°	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°			
12"	18"	1.3	2.6	3.9	5.2	1'-8"	1'-2"	3'-9"	1'-10"	1'-2"	0'-3"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.09	1.45	1.46	1.51	1.60	1.80	1.82	1.91	2.09	2.16	2.20	2.33	2.60	12"	18"	15"
14"	23"	1.8	3.6	5.4	7.2	1'-10"	1'-3"	4'-21/2"	1'-11"	1'-3"	8½"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	1.36	1.82	1.84	1.89	2.01	2.29	2.32	2.43	2.68	2.75	2.80	2.97	3.33	14"	23"	18"
19"	30"	3.3	6.6	9.9	13.2	2'-3"	1'-4"	5'-11/2"	2'-0"	1'-4"	1'-71/2"	4'-2"	4'-2"	4'-4"	4'-10"	5'-11"	1.89	2.55	2.57	2.65	2.82	3.22	3.27	3.43	3.77	3.88	3.95	4.19	4.70	19"	30"	24"
24"	38"	5.1	10.2	15.3	20.4	2'-8"	1'-5"	6'-3"	2'-1"	1'-5"	2'-9"	5'-2"	5'-2"	5'-4"	6'-0"	7'-4"	2.64	3.55	3.58	3.69	3.93	4.48	4.54	4.77	5.24	5.39	5.49	5.82	6.53	24"	38"	30"
29"	45"	7.4	14.8	22.2	29.6	3'-1"	1'-6"	7'-0"	2'-2"	1'-6"	3'-6"	6'-0"	6'-0"	6'-3"	6'-11"	8'-6"	3.32	4.48	4.52	4.66	4.96	5.64	5.72	6.00	6.60	6.80	6.92	7.34	8.24	29"	45"	36"
34"	53"	10.2	20.4	30.6	40.8	3'-6"	1'-7"	7'-111½"	2'-3"	1'-7"	4'-5½"	7'-1"	7'-1"	7'-4"	8'-2"	10'-0"	4.24	5.76	5.81	6.00	6.39	7.29	7.40	7.76	8.55	8.81	8.97	9.52	10.70	34"	53"	42"
38"	60"	12.9	25.8	38.7	51.6	3'-10"	1'-8"	8'-9"	2'-4"	1'-8"	5'-3"	7'-11"	7'-11"	8'-2"	9'-2"	11'-2"	5.22	7.16	7.23	7.46	7.96	9.10	9.24	9.70	10.71	11.05	11.25	11.95	13.46	38"	60"	48"
43"	68"	16.6	33.2	49.8	66.4	4'-3"	1'-10"	9'-81/2"	2'-6"	1'-10"	6'-2 ¹ / ₂ "	8'-10"	8'-10"	9'-2"	10'-2"	12'-6"	6.63	9.01	9.09	9.38	10.00	11.39	11.56	12.13	13.36	13.77	14.02	14.88	16.73	43"	68"	54"
48"	76"	20.5	41.0	61.5	82.0	4'-8"	2'-1"	10'-8"	2'-9"	2'-0"	7'-2"	9'-9"	9'-9"	10'-1"	11'-3"	13'-9"	8.66	11.74	11.85	12.22	13.02	14.82	15.04	15.77	17.37	17.91	18.23	19.34	21.74	48"	76"	60"
53"	83"	24.8	49.6	74.4	99.2	5'-1"	2'-6"	11'-7"	3'-2"	2'-6"	8'-1"	10'-7"	10'-7"	10'-11"	12'-3"	15'-0"	12.50	16.98	16.98	17.67	18.83	21.47	21.78	22.86	25.18	25.97	26.44	28.06	31.55	53"	83"	66"
58"	91"	29.5	59.0	88.5	118.0	5'-6"	2'-10"	12'-6 ¹ / ₂ "	3'-6" .	2'-10"	9'-01/2"	11'-4"	11'-4"	11'-9"	13'-1"	16'-0"	16.46	22.26	22.46	23.16	24.66	28.05	28.46	29.85	32.85	33.85	34.46	36.55	41.05	58"	91"	72"
																															1	

REVISION 11/01/17

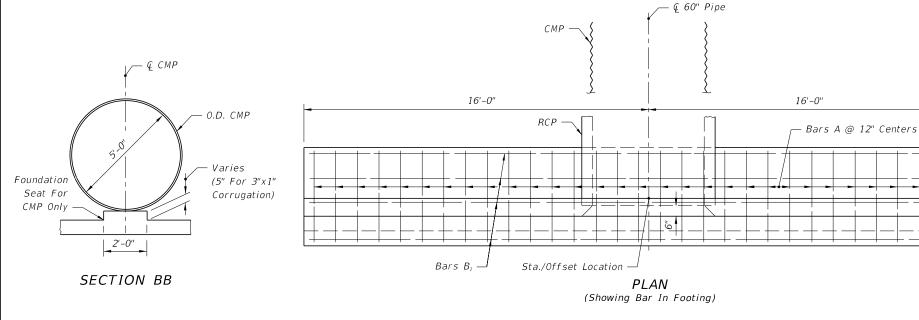
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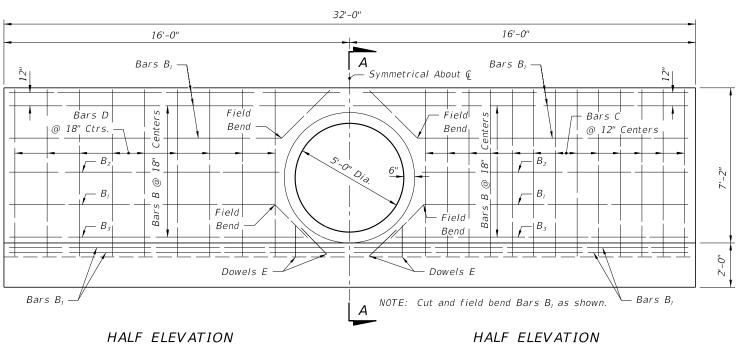
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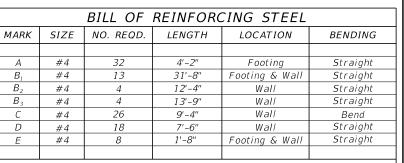
FY 2019-20 STANDARD PLANS

INDEX 430-030

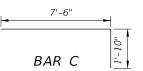
SHEET 2 of 2







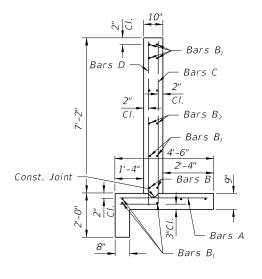
### BENDING DIAGRAM



NOTE: All bar dimensions are out to out

## ESTIMATED QUANTITIES

ITEM	UNIT	RCP	СМР
Class II Concrete	Cu. Yd.	11.3	11.4
Reinforcing Steel	Lb.	695	695



TYPICAL SECTION THRU ENDWALL

### GENERAL NOTES

(Showing Bars In Back Face Of Wall)

- 1. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this Index. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index 425-001 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.

(Showing Bars In Front Face Of Wall)

4. Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

- 5. Chamfer: All exposed edges and corners to be chamfered 3/4" unless otherwise shown.
- 6. That portion of corrugated metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of .004" minimum thickness applied prior to placing of the concrete.
- 7. Sodding shall be in accordance with Index 524-001 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

**REVISION** 11/01/17

(3"x1" Corr.)

(Class B Wall) -

Bars B

В

SECTION AA

OPTIONAL ENTRANCE

FOR CONCRETE PIPE

(See Option Below)



FY 2019-20 STANDARD PLANS

STRAIGHT CONCRETE ENDWALLS

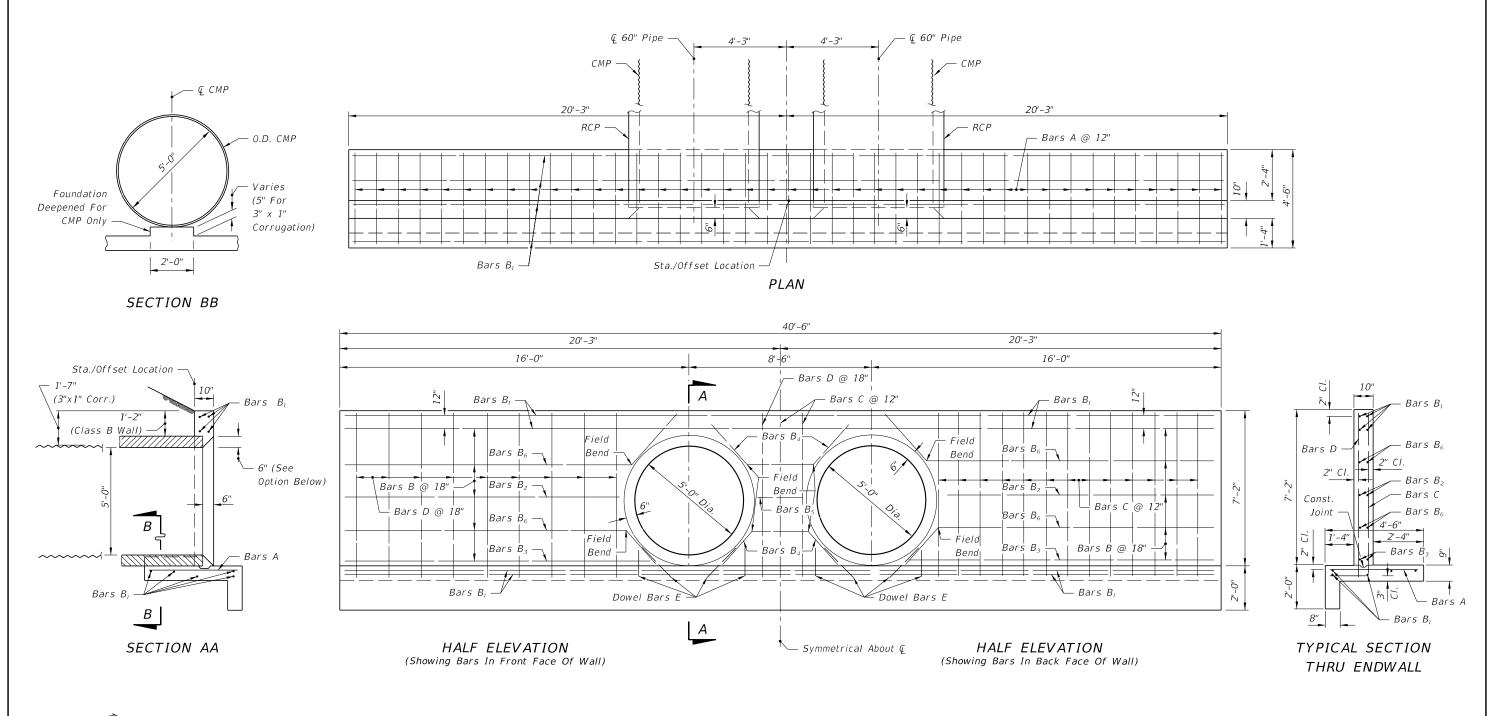
INDEX

SHEET 1 of 2

DESCRIPTION:

SINGLE AND DOUBLE 60" PIPE

430-031





DESCRIPTION:

OPTIONAL ENTRANCE FOR CONCRETE PIPE

		BILL OF	REINFO	RCING STEEL	
MARK	SIZE	NO. REQD.	LENGTH	LOCATION	BENDING
Α	#4	41	4'-2"	Footing	Straight
$B_1$	#4	9	40'-2"	Footing & Wall	Straight
$B_2$	#4	4	12'-6"	Wall	Straight
Вз	#4	4	13'-9"	Wall	Straight
$B_4$	#4	4	6'-0"	Wall	Field Bend
$B_5$	#4	2	2'-2"	Wall	Straight
$B_6$	#4	8	15'-0"	Wall	Field Bend
С	#4	29	9'-4"	Footing & Wall	Bend
D	#4	20	7'-6"	Footing & Wall	Straight
Ε	#4	16	1'-8"	Footing & Wall	Straight

	BENDING	DIAGRA	М	
	7'-6	" <b>-</b>		
	<u>'</u>	= =	Γ	
		-10		
	BAR	- 1	L	,
_	NOTE: All bar di			JT
,	ESTIMATED	QUANT I	TIES	
_	ITEM	UNIT	RCP	CMP
4	Class II Concrete	Cu. Yd.	13.7	13.8
_	Reinforcing Steel	Lb.	824	824

NDTE: See Sheet 1 of 2 For General Notes.

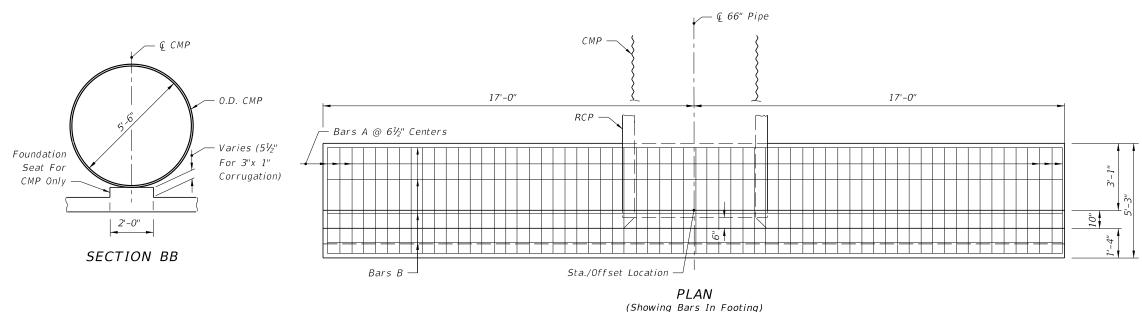
REVISION 11/01/17

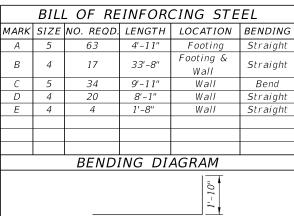
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FY 2019-20 STANDARD PLANS

INDEX 430-031

SHEET 2 of 2

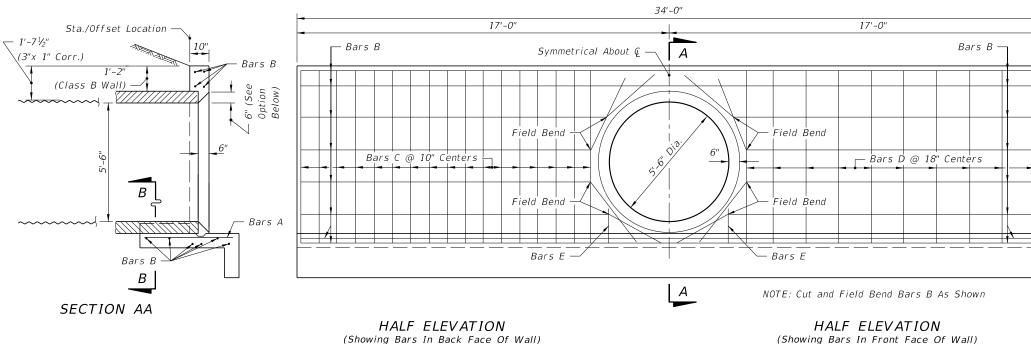




8'-1" BAR C

NOTE: All bar dimensions are out to out

ESTIMATED QUANTITIES							
ITEM	UNIT	RCP	CMP				
Class II Concrete	Cu. Yd.	13.2	13.3				
Reinforcing Steel	Lb.	1170	1170				



(Showing Bars In Front Face Of Wall)





OPTIONAL ENTRANCE FOR CONCRETE PIPE

DESCRIPTION:

- 1. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this Index. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index 425-001 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

### GENERAL NOTES

- 5. Chamfer: All exposed edges and corners to be chamfered  $rac{3}{4}$ " unless otherwise shown.
- 6. That portion of corrugated Metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of 0.004" minimum thickness applied prior to placing of the concrete.

Const. Joint

- 7. Sodding shall be in accordance with Index 524-001 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

**REVISION** 

FY 2019-20 STANDARD PLANS

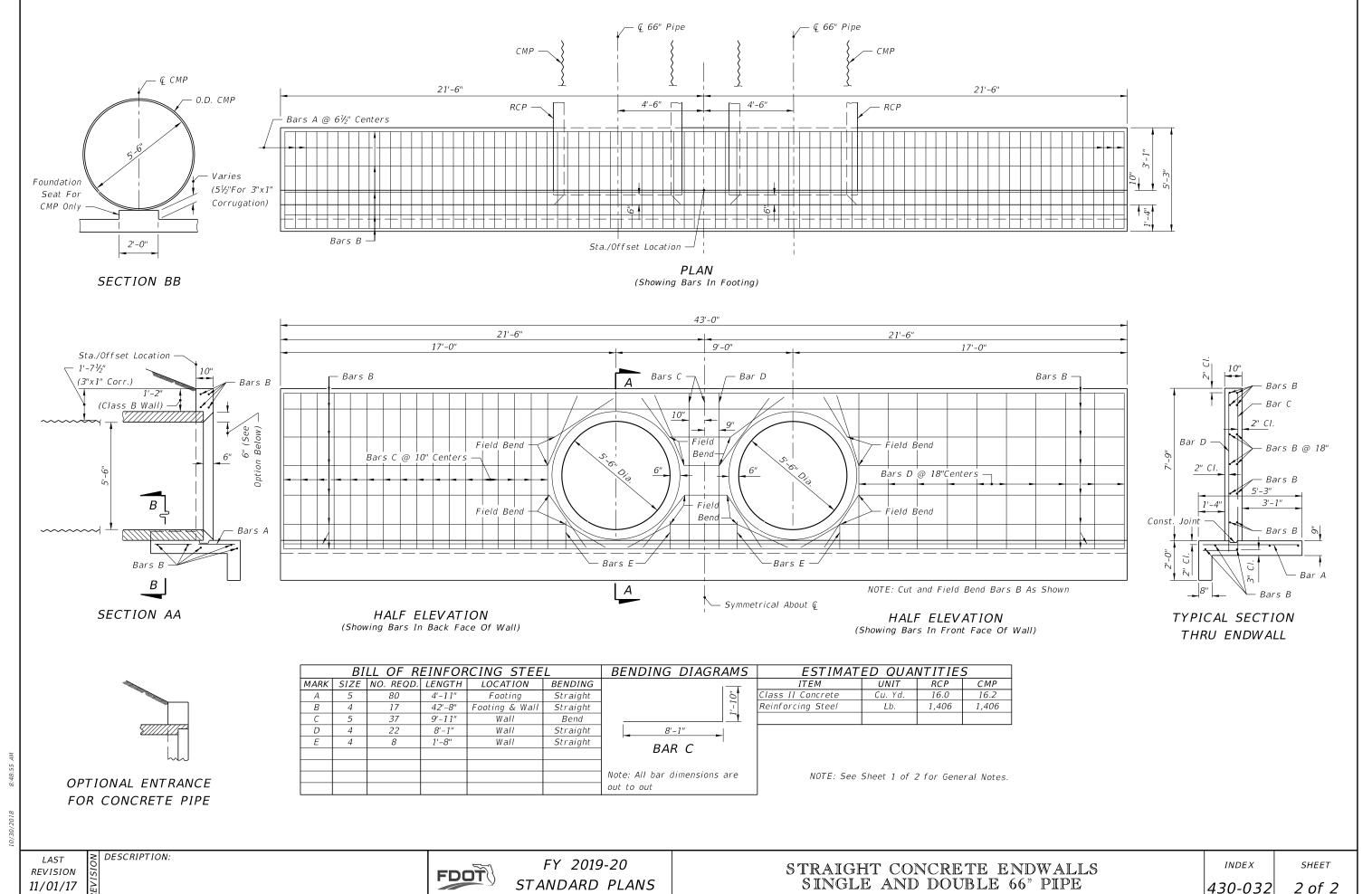
STRAIGHT CONCRETE ENDWALLS

INDEX 430-032

SHEET

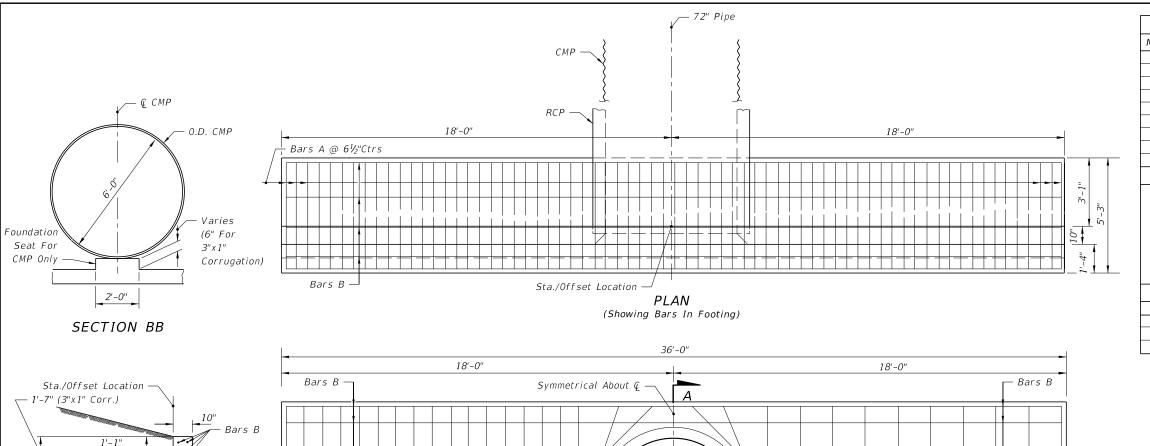
11/01/17

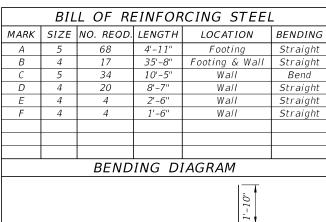
SINGLE AND DOUBLE 66" PIPE



STANDARD PLANS

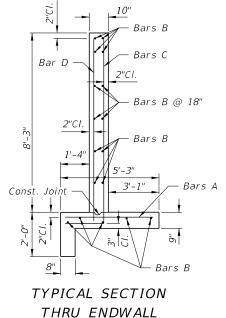
SINGLE AND DOUBLE 66" PIPE

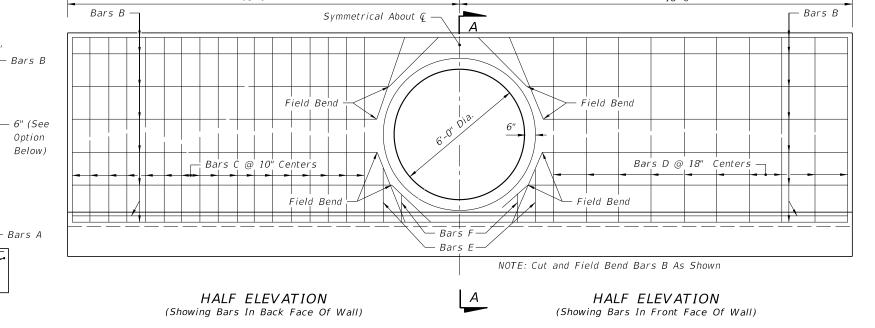






ESTIMATED QUANTITIES						
ITEM	UNIT	RCP	CMP			
Class II Concrete	Cu. Yd.	14.4	14.5			
Reinforcing Steel	Lb.	1249	1249			





## GENERAL NOTES

- 1. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this Index. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index 425-001 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

- 5. Chamfer: All exposed edges and corners to be chamfered ¾" unless otherwise shown.
- 6. That portion of corrugated Metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of 0.004" minimum thickness coated applied prior to placing of the concrete.
- 7. Sodding shall be in accordance with Index 524-001 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

**REVISION** 11/01/17

(Class B_Wall) -

В

В

SECTION AA

OPTIONAL ENTRANCE

FOR CONCRETE PIPE

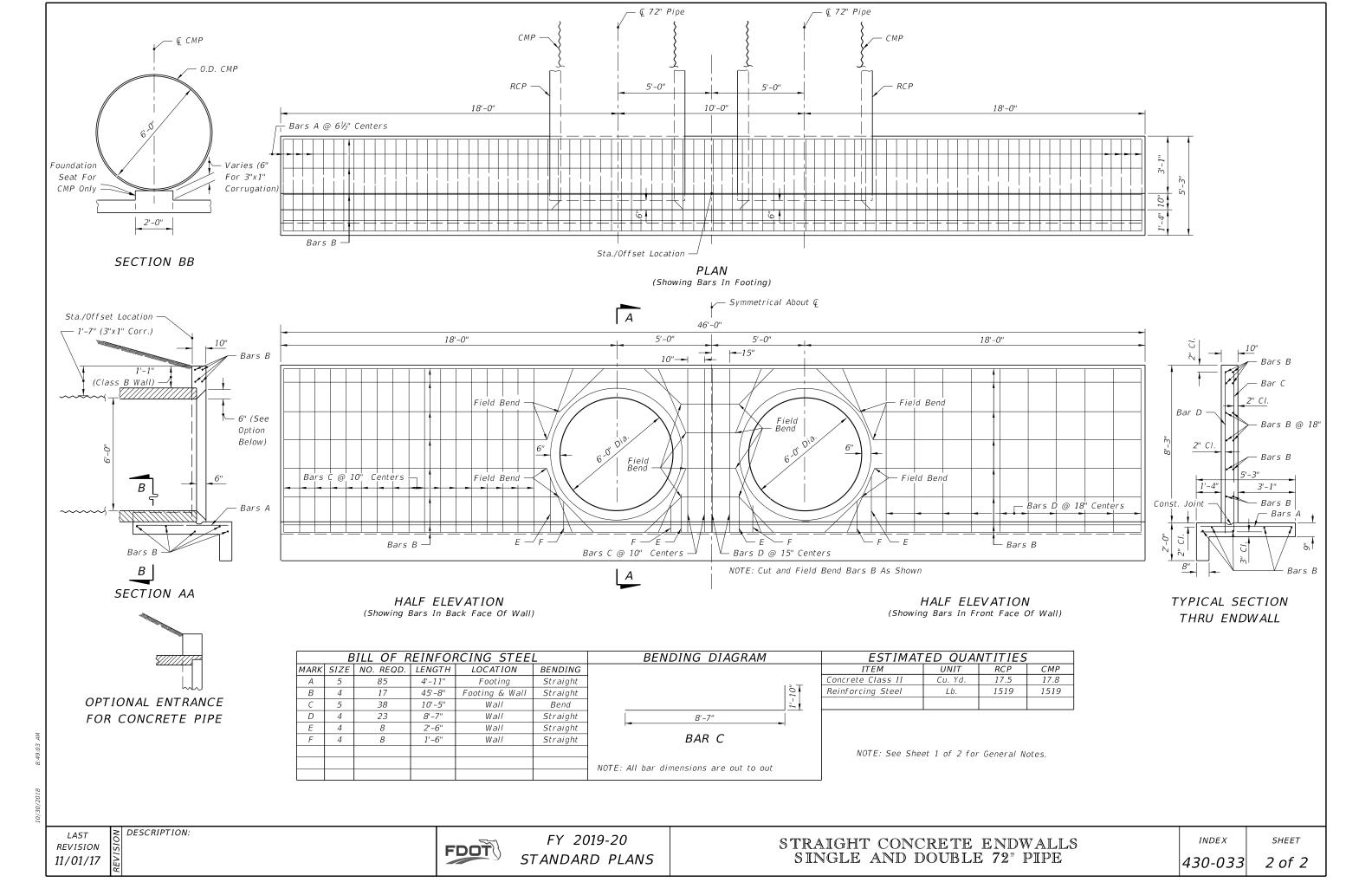
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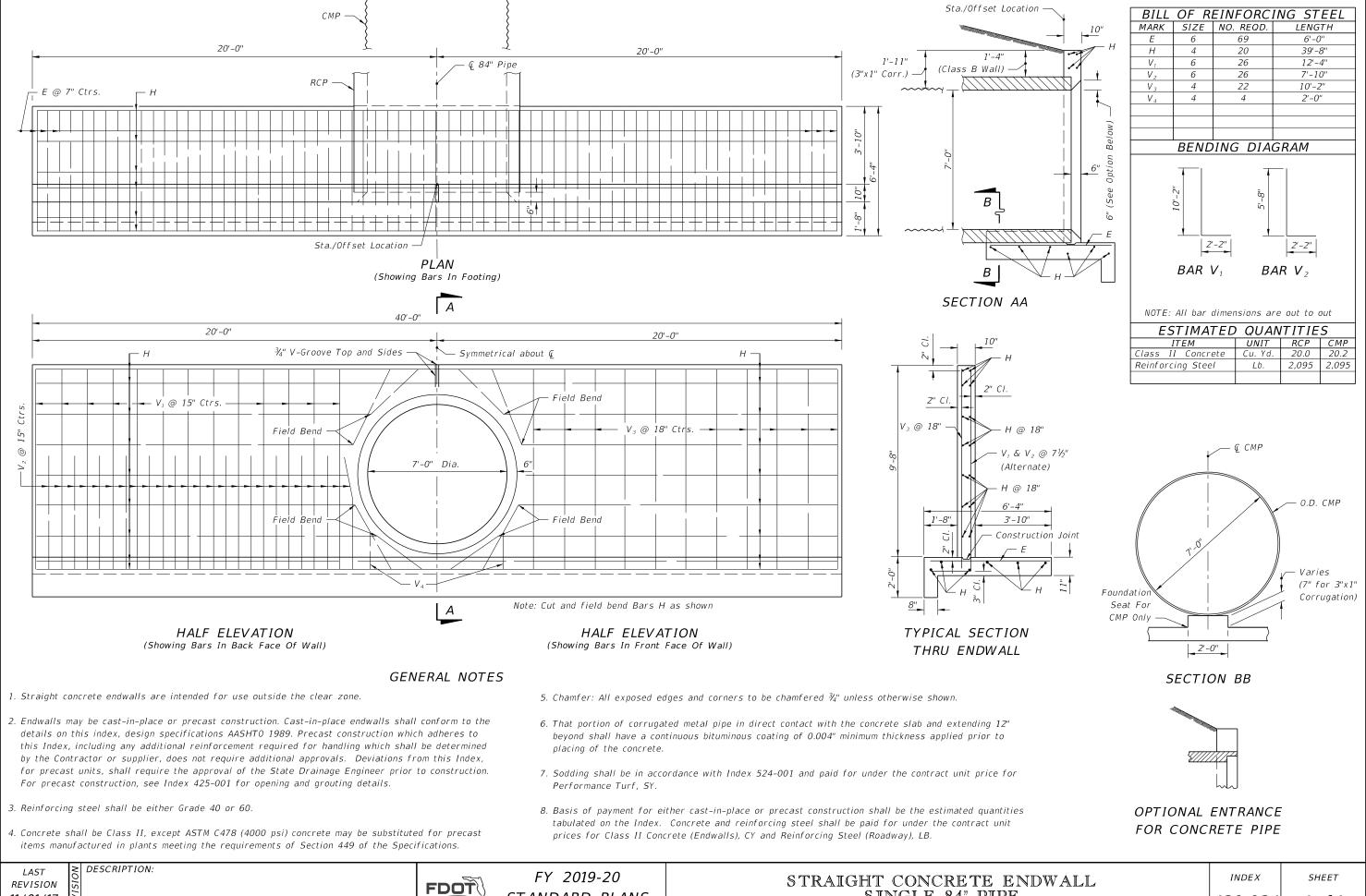
FY 2019-20 STANDARD PLANS

STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 72" PIPE

INDEX

SHEET





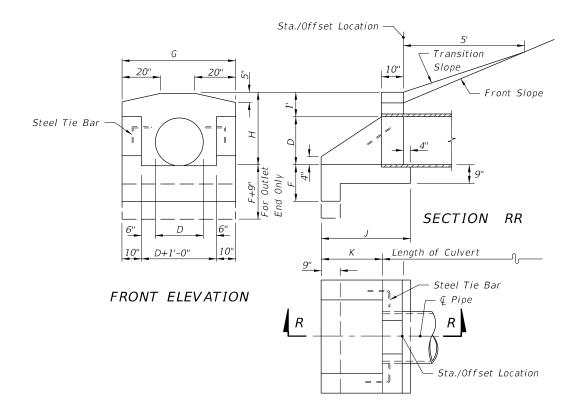
11/01/17

STANDARD PLANS

SINGLE 84" PIPE

430-034

1 of 1



PLAN CONCRETE ENDWALL WITH U-TYPE WINGS FOR PIPE CULVERTS

## TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH U-TYPE WINGS

	DIMENSIONS							QUANTITIES IN ONE ENDWALL					
Ор	Opening Wall Footing		ting	Concrete, Class I, Total (CY)						Steel			
D	Area	G	Н	Κ	F	1	R	CP	С	MP	CIP		Tie Bars
	(ft²)	G	г	~	F	J	Inlet	Outlet	Inlet	Outlet	Inlet	Outlet	THE Dats
12"	0.8	3'-8"	2'-0"	1'-0"	1'-3"	2'-2"	0.48	0.55	0.49	0.57	0.49	0.57	none
15"	1.2	3'-11"	2'-3"	1'-5"	1'-3"	2'-7"	0.59	0.67	0.62	0.70	0.61	0.70	none
18"	1.8	4'-2"	2'-6"	1'-9"	1'-3"	2'-11"	0.70	0.79	0.74	0.82	0.74	0.82	none
24"	3.1	4'-8"	3'-0"	2'-6"	1'-6"	3'-8"	1.01	1.11	1.06	1.16	1.06	1.16	2-#6 Bars x 2'-0"
30"	4.9	5'-2"	3'-6"	3'-3"	1'-6"	4'-5"	1.33	1.44	1.41	1.51	1.40	1.51	2-#6 Bars x 2'-0"
36"	7.1	5'-8"	4'-0"	4'-0"	1'-9"	5'-2"	1.73	1.85	1.84	1.96	1.82	1.94	2-#6 Bars x 2'-6"
42"	9.6	6'-2"	4'-6"	4'-9"	2'-0"	5'-11"	2.19	2.32	2.32	2.45			2-#6 Bars x 2'-6"
48"	12.6	6'-8"	5'-0"	5'-6"	2'-0"	6'-8"	2.64	2.78	2.81	2.95			2-#6 Bars x 3'-0"

# Sta./Offset Location - Transition Slope Front Slope Steel Tie Bar 1'-6" FRONT ELEVATION SECTION NN Length of Culvert — Sta./Offset Location Steel Tie Bar PLAN

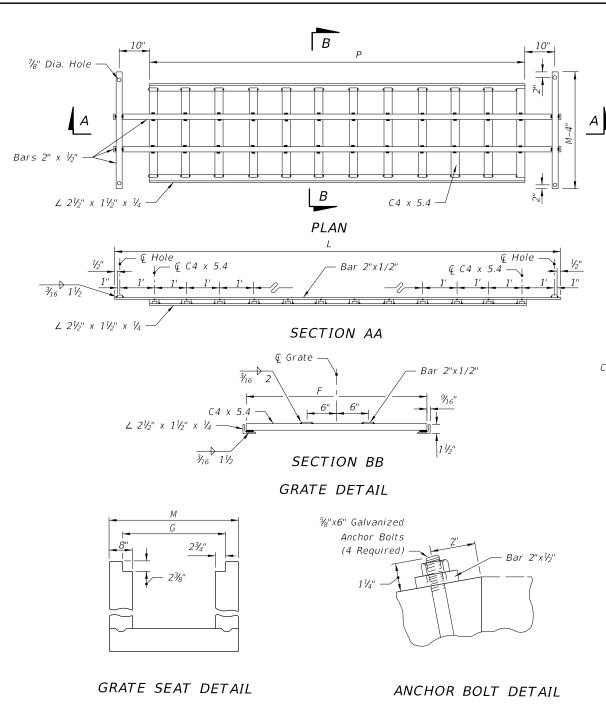
## CONCRETE ENDWALL WITH 45° WINGS FOR PIPE CULVERTS

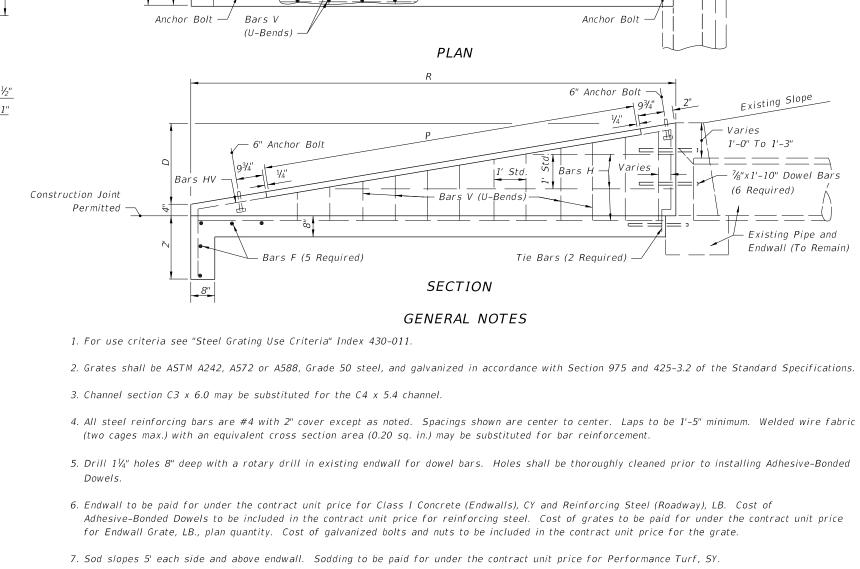
## TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH 45° WINGS

			DIMENSI	ONS			QUANTITIES IN ONE ENDWALL			
Opening Wall				Footing	Concre	te, Class				
D	Area	н	G	,	м	F	Total (CY)			Steel Tie Bars
	(ft²)	П	G		141		RCP	CMP	CIP	
15"	1.2	2'-3"	3'-7"	1'-0"	1'-3"	1'-3"	0.56	0.59	0.59	none
18"	1.8	2'-6"	3'-10"	1'-2"	1'-7"	1'-3"	0.74	0.77	0.77	none
24"	3.1	3'-0"	4'-4"	1'-5"	2'-1"	1'-4"	1.01	1.06	1.06	2 -#6 Bars x 2'-0"
30"	4.9	3'-6"	4'-10"	1'-9"	2'-5"	1'-6"	1.32	1.40	1.39	2 -#6 Bars x 2'-0"
36"	7.1	4'-0"	5'-4"	2'-0"	2'-11"	1'-8"	1.72	1.83	1.82	2 -#6 Bars x 2'-6"
42"	9.6	4'-6"	5'-10"	2'-3"	3'-6"	2'-0"	2.34	2.47		2 -#6 Bars x 2'-6"
48"	12.6	5'-0"	6'-4"	2'-6"	4'-0"	2'-0"	2.74	2.90		2 -#6 Bars x 2'-6"

## GENERAL NOTES

- 1. Winged concrete endwalls are intended for use outside the clear zone.
- 2. Chamfer all exposed edges ¾".
- 3. Concrete shall be Class I, except ASTM C478 (4000 psi) Concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 4. Endwall to be paid for under the contract unit price for Class I Concrete.
- 5. Sodding to be in accordance with Index 524-001, and paid for under the contract unit price for Performance Turf, SY.





			DIMEN	ISIONS	S AND C	UANTI	TIES PE	R GRATE		
Slope	Pipe	Channels	@ 5.4 Lb	s./LF	Bars @ 3.4 lbs/LF (2 ea.)			Angles @	3.2 Lbs./LF	(2)Total
Stope	Size	Quantity	F	Lbs.	L	M-4"	Lbs.	Р	Lbs.	Weight-Lbs
	15"	10	2'-67/8"	139	11'-3"	3'-3"	99	9'-4"	60	298
1:6	18"	12	2'-97/8"	183	13'-3"	3'-6"	114	11'-4"	73	370
1.0	24"	15	3'-37/8"	269	16'-3"	4'-0"	138	14'-4"	92	499
	30"	18	3'-97/8"	<i>372</i>	19'-3"	4'-6"	162	17'-4"	111	645
	15"	6	2'-67/8"	83	7'-3"	3'-3"	71	5'-4"	34	188
1:4	18"	7	2'-91/8"	107	8'-3"	3'-6"	80	6'-4"	41	228
1.4	24"	9	3'-37/8"	161	10'-3"	4'-0"	97	8'-4"	53	311
	30"	11	3'-97/8"	227	12'-3"	4'-6"	114	10'-4"	66	407

	D	IMENSI	ONS AI	ND QUA	ANTITIE	S PER U-E	NDWALL	
Pipe Size	G	М	D	R	Р	Class I Concrete-CY	Reinforcing Steel-Lbs.	Sod SY
15"	2'-81/2"	3'-7"	2'-2"	13'-0"	9'-4"	2.12	167	23
18"	2'-111/2"	3'-10"	2'-5"	14'-6"	11'-4"	2.53	173	25
24"	3'-5½"	4'-4"	2'-11"	17'-6"	14'-4"	3.48	238	29
30"	3'-111/2"	4'-10"	3'-5"	20'-6"	17'-4"	4.57	315	32
15"	2'-81/2"	3'-7"	2'-2"	8'-8"	5'-4"	1.44	120	19
18"	2'-111/2"	3'-10"	2'-5"	9'-8"	6'-4"	1.72	130	20
24"	3'-5 ¹ / ₂ "	4'-4"	2'-11"	11'-8"	8'-4"	2.36	167	22
30"	3'-11½"	4'-10"	3'-5"	13'-8"	10'-4"	3.09	225	25

Grate Seat

1' Std.

PLAN

SECTION

GENERAL NOTES

Anchor Bolt

6" Anchor Bolt

Bars H

Tie Bars (2 Required)

Existing Slope

7/8"x1'-10" Dowel Bars

Existing Pipe and Endwall (To Remain)

(6 Required)

Varies 1'-0" To 1'-3"

**REVISION** 11/01/17

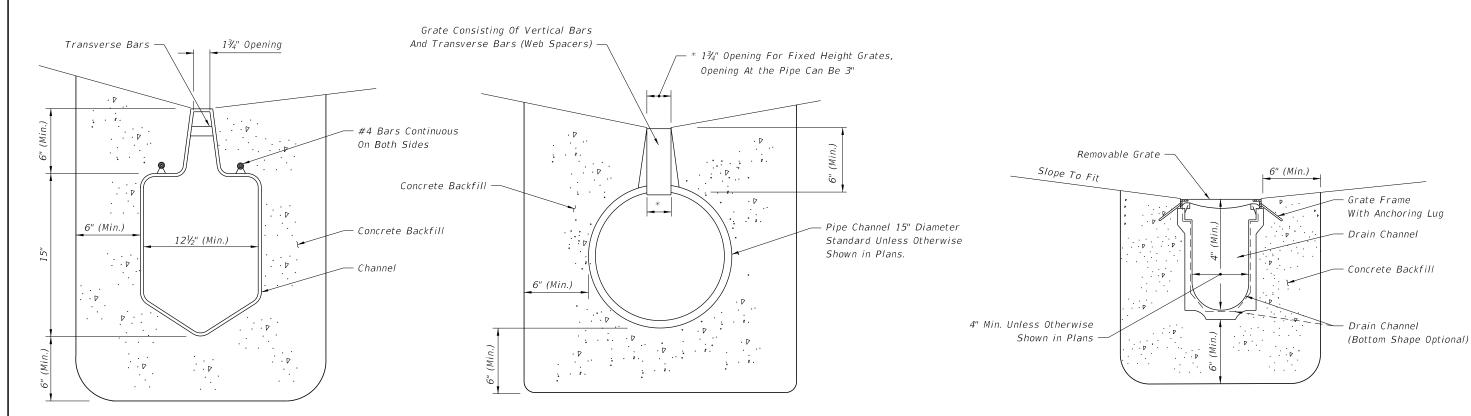
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS INDEX

SHEET 1 of 1

SAFETY MODIFICATIONS FOR ENDWALLS



PREFORMED POLYETHYLENE ALTERNATE

ROUND ALTERNATE

SEE SHEET 2 FOR TYPICAL LOCATIONS

# TYPE I (NON-REMOVABLE GRATE)

#### GENERAL NOTES

- 1. Trench drain is intended for use in gutters and driveways as shown on the typical locations on Sheet 2. Type I is intended for use in Type E, F and drop curbing, and adjacent to traffic separators and standard barrier walls. The width of the channel grate for Type I Trench Drain shall be 1¾" throughout varying the depth of the channel neck. Type II may also be used in those locations if an independent laboratory certifies that the grating used has an open area equal to at least 0.27 square feet per linear foot. Type II is primarily intended for use in valley gutter across driveway openings and drop curbing; Type I may also be used in those locations. The width of the channel grate for Type II Trench Drain shall be the same as the width of the channel. The linear slope or gradient for Type II may be manufactured by varying the depth of the channel. Trench Drain shall not be placed in pedestrian paths unless ADA compliant grates are used.
- 2. Unless shown in the plans, outlet pipes and preformed channel inverts shall be sloped 0.6% or steeper toward the outlet regardless of the surface slope.
- 3. Trench drain may be stubbed directly into drainage structures, or outlet pipes may be used to connect trench drain to drainage structures.
- 4. A cleanout port compatible with the manufactured system shall be provided for Type I drains at the upstream end and at intervals not to exceed 50 feet. The cleanout port shall provide an opening 6" to 10" wide (transverse to the trench drain length) and 18" to 24" long. Where cleanouts are placed adjacent to raised curb or separator, the curb or separator shall be formed around the cleanout. The cleanout shall have a removable load resistant cover or grate.
- 5. Trench excavation must allow for a minimum of 6" of concrete to be placed under and alongside the trench drain channel system. Concrete backfill shall meet the requirements of Section 347 of the Standard Specifications. At the end of all units (Type I or II), the concrete backfill shall extend 6" minimum past the end of the drain opening.
- 6. Transverse bars for Type I Trench Drain shall be spaced 4" to 6" on center.
- 7. Whenever the work disturbs existing conditions or work already completed, restore the same to its original condition in every detail. All such repair and replacement shall meet the approval of the Engineer.

## PREFORMED CHANNEL WITH REMOVABLE GRATE

SEE SHEET 2 FOR TYPICAL LOCATIONS

# TYPE II

#### DESIGN NOTES

- 1. Where placed adjacent to reinforced concrete barrier, designer shall detail in the plans the position of the drain relative to the barrier to avoid conflicts with the foundation. (See Index 521-001)
- 2. The designer shall identify the following in the plans:
  - (a) The type of drain at each location.
  - (b) The begin and end locations of the Trench Drain.
  - (c) The location of the outlet pipe if the Trench Drain is not stubbed directly into a drainage structure.
  - (d) The design flow (Q) for the Trench Drain must be shown on the plans.
- 3. Capture efficiency for Type I Trench Drain may be computed using the equations for slotted drain in FHWA's HEC 12 & 22. Grate Type I and Type II must have at least 30% open area.
- 4. Round pipe alternate is available in 12, 18, 24 and 36 inch.
- 5. Type II Preformed Channel with integral anchoring lugs are applicable.

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LAST REVISION 11/01/17

DESCRIPTION:

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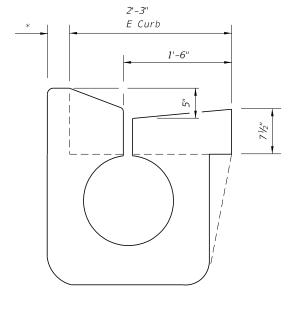
FY 2019-20 STANDARD PLANS

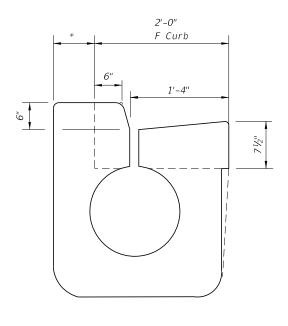
TRENCH DRAIN

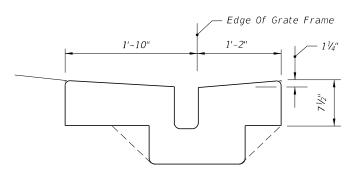
INDEX

SHEET

436-001



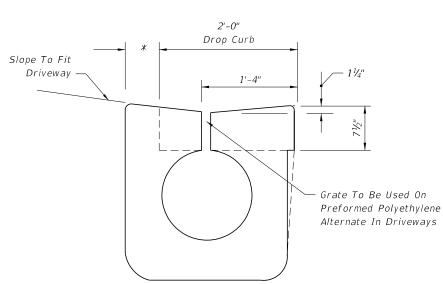




WITHIN VALLEY GUTTER

WITHIN TYPE E CURB

WITHIN TYPE F CURB



WITHIN DROP CURB

On hylene eways

Slope To Fit Driveway —
Separator

*

Slope To Fit Roadway

TYPI

WITHIN DROP CURB
TYPICAL LOCATIONS FOR TYPE II

ADJACENT TO TRAFFIC SEPARATOR

* As Necessary To Provide 6" Of Concrete On This Side Of Drain

ROUND PIPE ALTERNATE SHOWN, BUT PREFORMED POLYETHYLENE ALTERNATE ACCEPTABLE

TYPICAL LOCATIONS FOR TYPE I

LAST O DESCRIPTION:
REVISION 11/01/17

FDOT

FY 2019-20 STANDARD PLANS

TRENCH DRAIN

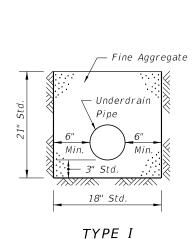
INDEX

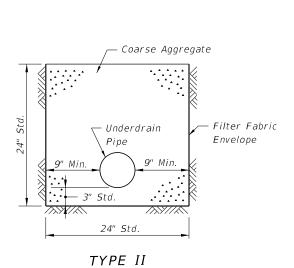
- Edge Of Grate Frame

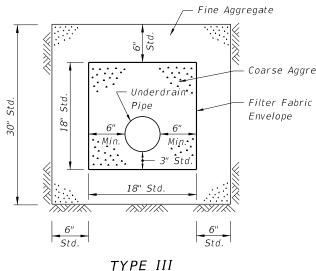
Optional Trench

SHEET

436-001 2 of 2







## **DESIGN NOTES**

- 1. The type of underdrain should be selected to meet design water removal rate and soil conditions. Caution is prescribed in the use of these typical sections since special designs may be required to satisfy project conditions.
- 2. Type I underdrain is intended for minimum water removal conditions.
- 3. Type II underdrain is intended for moderate water removal conditions. Where reactive conditions may create chemical clogging, the use of an inert material and/or elimination of the filter fabric may be necessary.
- 4. Type III underdrain is intended for maximum water removal conditions. Filter fabric is required between the coarse aggregate or fine aggregate including those described in general notes 2 and 3. Design note 3 applies for reactive conditions.
- 5. Type V underdrain is intended for use in detention basins and other locations which require a filtration system. The standard fine aggregate specified for Type V underdrain conforms to filtration gradation requirements of Chapter 62–25 FAC.
- 6. The designer should detail in the plans, the location of:
  (a) Type V underdrain, (b) nonstandard locations of Type I, II, and III underdrain, (c) underdrain inspection boxes, (d) cleanouts for Type V underdrain, and (e) underdrain outlet pipes.
- 7. The designer should specify the flow line elevations at the beginning, bends, junctions and ends of underdrain pipes and outlet pipes.
- 8. The designer should evaluate whether an external filter fabric envelope is required around underdrain Types I and III. When required, fabric shall be specified in the plans.

#### GENERAL NOTES

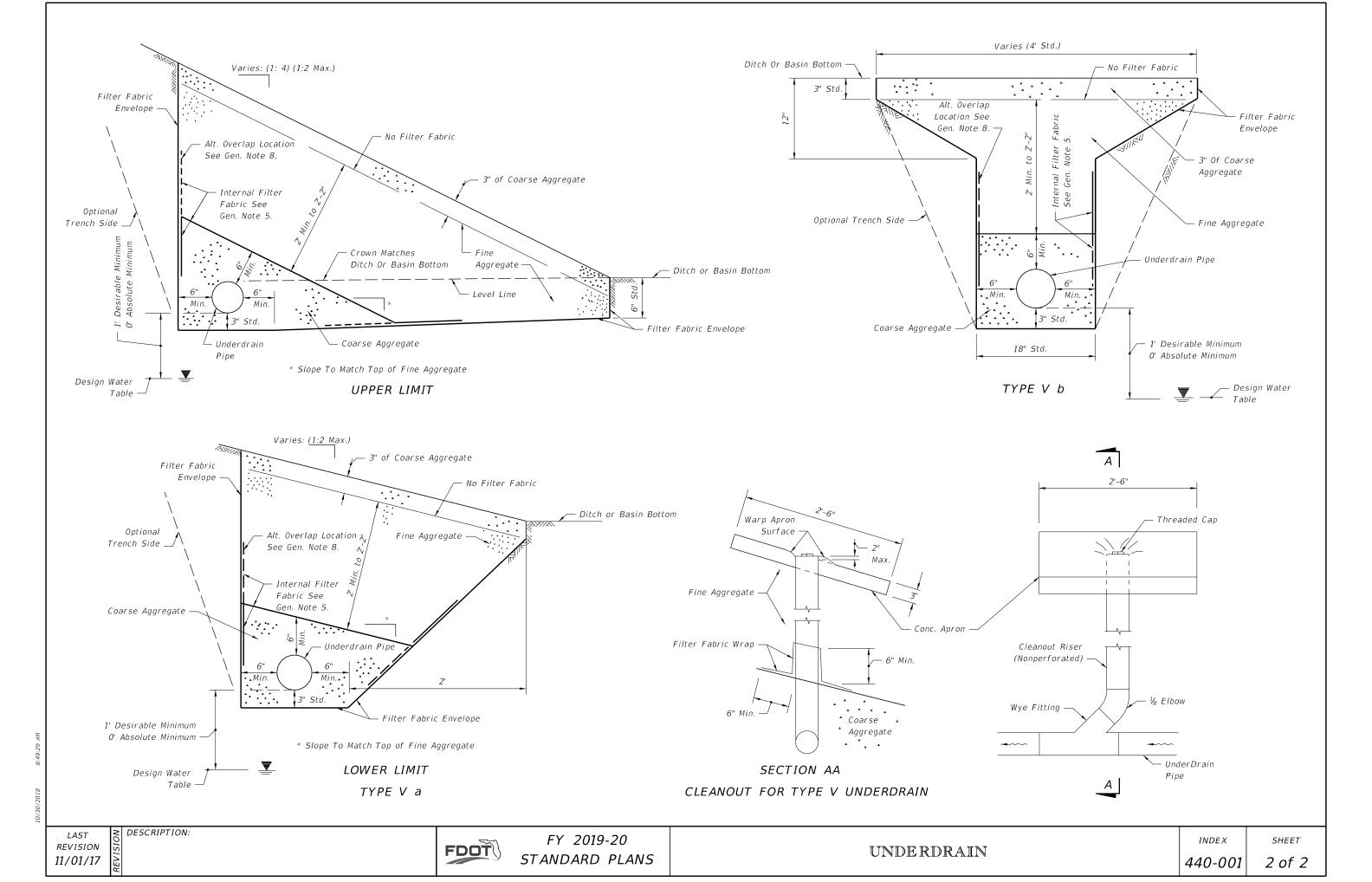
- 1. The underdrain pipe shall be either 4" smooth or 5" corrugated tubing unless otherwise shown in the plans. The size to be furnished will be based on the nominal internal diameter of a pipe with a smooth interior wall. Except when prohibited by the plans, the special provisions or this standard, pipe with a corrugated interior wall may be provided based on the following size equivalency.
  - 4" smooth interior equivalent to 5" corrugated interior
  - 5" smooth interior equivalent to 6" corrugated interior
  - 6" smooth interior equivalent to 8" corrugated interior
  - 8" smooth interior equivalent to 10" corrugated interior
- 2. Fine aggregate shall be quartz sand meeting the requirements of Sections 902-4 of the Standard Specifications.
- 3. Coarse aggregate shall be gravel or stone meeting the requirements of Sections 901–2 or 901–3. The gradation shall meet Section 901, Grades 4, 467, 5, 56 or 57 stone unless otherwise shown restricted in the plans.
- 4. Underdrain Type I, II, III and V shall be in accordance with Section 440.
- 5. Filter fabric shall be Type D-3 (See Specifications Section 985). The internal filter fabric of Type V underdrain shall have a permittivity of 0.7 /sec. and an AOS of #40 sieve.
- 6. When Type I is used, a filter fabric sock meeting Section 948 is required.
- 7. See Index 120-002 for the standard location of Type I, II, and III underdrain. The location of Type V underdrain and nonstandard locations of Type I, II, and III underdrain will be as detailed in the plans.
- 8. All filter fabric joints shall overlap a minimum of 1'. The internal filter fabric of Type V underdrain shall overlap into the coarse aggregate or the fine aggregate a minimum of 1'.
- 9. Underdrain outlet pipes shall be nonperforated and all bends shall be made using ½ (45 deg.) elbows. 90 deg. bends shall be constructed with two ½ elbows separated by at least 1' of straight pipe. Outlet pipes stubbed into inlets or other drainage structures shall be not less than 6" above the structure flow line. Outlet pipes discharging to grassed areas shall have concrete aprons, hardware cloth, and bordering sod as shown in Index 466-001 for Education outlets.
- 10. Pay Item shall be based on the size of the smooth interior products. The contract unit price for Underdrain, LF, shall include the cost of pipe, fittings, aggregate, sock, filter fabric, underdrain cleanouts, and concrete aprons.

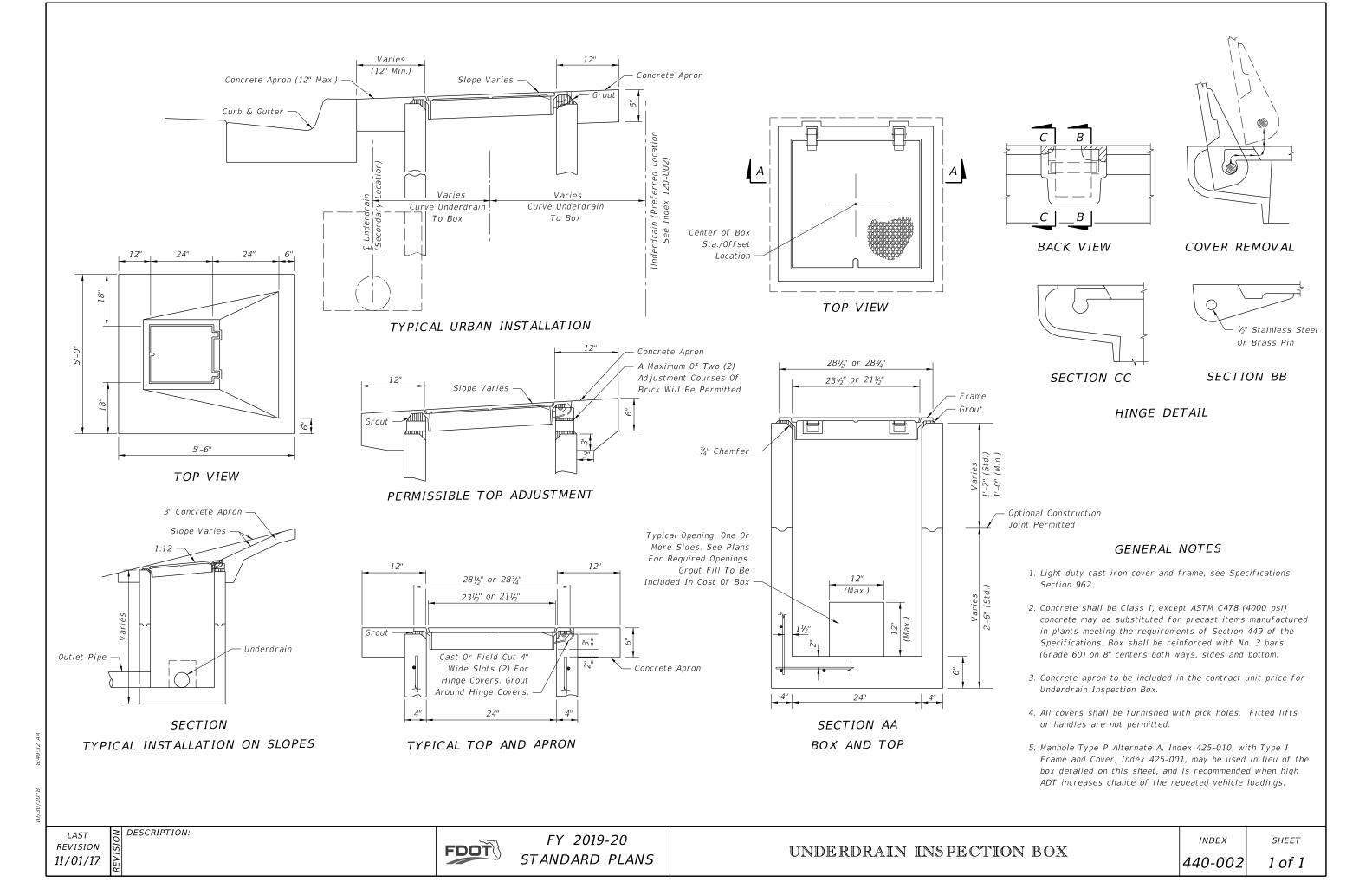
The contract unit price for Underdrain Outlet Pipe, LF, shall be full compensation for trench excavation, pipe and fittings, concrete aprons, hardware cloth for concrete aprons, stubbing into drainage structures, backfill in place, and disposal of excess materials.

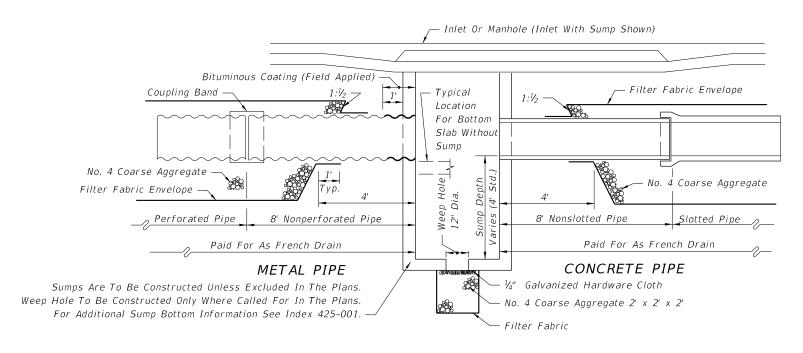
The contract unit price for Underdrain Inspection Box, EA. shall be for the number completed and accepted.

DESCRIPTION:

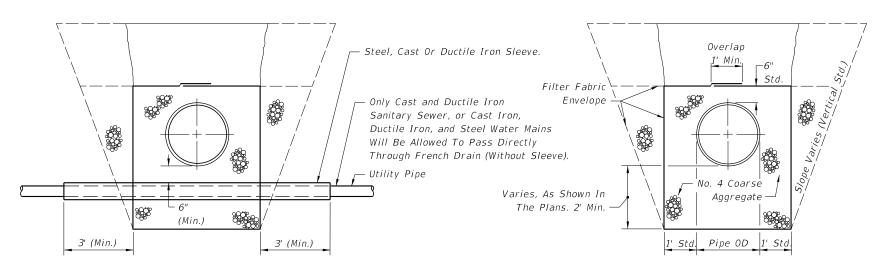
1 of 2







# LONGITUDINAL SECTION



ROUND PIPE SHOWN
UTILITY PIPES THRU FRENCH DRAIN

ROUND PIPE SHOWN
STANDARD CROSS SECTION (ENLARGED)

# FRENCH DRAIN SYSTEM

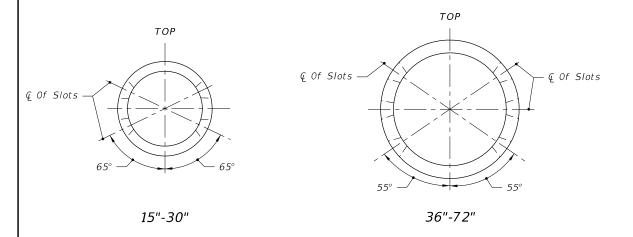
## GENERAL NOTES

- 1. Pipe shall be any of the optional types permitted in Section 443 of the Specifications unless otherwise restricted in the plans. Dissimilar types of pipe will not be permitted in a continuous run of pipe.
- 2. Concrete pipe shall be placed with the slots positioned on sides.
- 3. Alignment joints are standard (gaskets not required). Recorrugation of metal pipe ends not required.
- 4. The contractor may submit other methods of providing slots having equal or greater area of opening, for approval by the Engineer.
- 5. Filter fabric shall be Type D-3 meeting the requirements of Section 985. All filter fabric joints shall lap a minimum of one (1) foot.
- 6. The standard cross section shall be constructed unless other section(s) described or detailed in the plans.
- 7. For supplemental details see Index 430-001.
- 8. The contractor shall take the necessary precautions to prevent contamination of the trench with sand, silt and foreign materials.
- 9. French drains shall be paid for under the contract unit price for French Drains, LF. The unit price shall include the cost of pipe, pipe plugs, pipe fittings, coarse aggregate and filter fabric in place, and the cost for trench excavation, backfill and compaction. The unit price shall also include the cost for disposal of surplus excavated materials and cost for restoration of pavement removed or damaged by french drain construction, but shall not include payments for items paid for elsewhere.

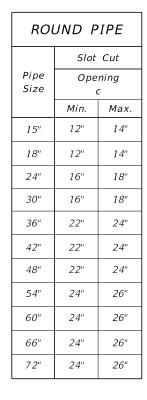
# **DESIGN NOTES**

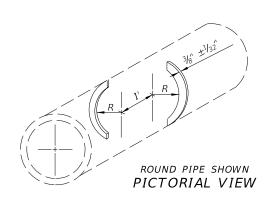
- 1. Pipe invert should be at or above the water table whenever possible.
- 2. French drains with minor dimensional changes or otherwise different from the standard cross-section shall be either described or detailed in the plans. French drains with significantly different cross-sections shall be detailed in the plans.

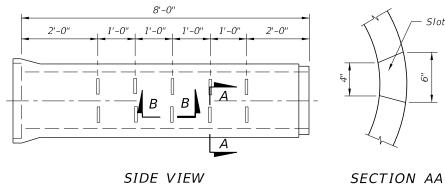
8.40:3

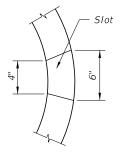


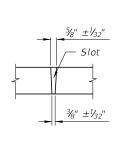
ELLIPTICAL PIPE				
	Slot	Cut		
Pipe Size	Opening c			
	Min.	Max.		
14"x23"	10"	12"		
19"x30"	14"	16"		
24"x38"	14"	16"		
29"x45"	20"	22"		
34"x53"	20"	22"		
38"x60"	20"	22"		



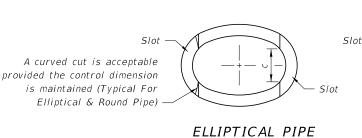


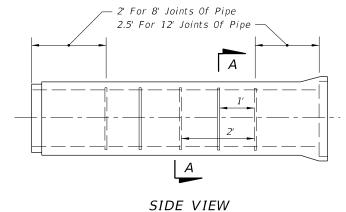






SECTION BB





OPTION A - ROUND PIPE

ROUND PIPE SECTION AA

OPTION B - ROUND OR ELLIPTICAL PIPE

SLOTTED PIPE OPTIONS

REVISION 11/01/17

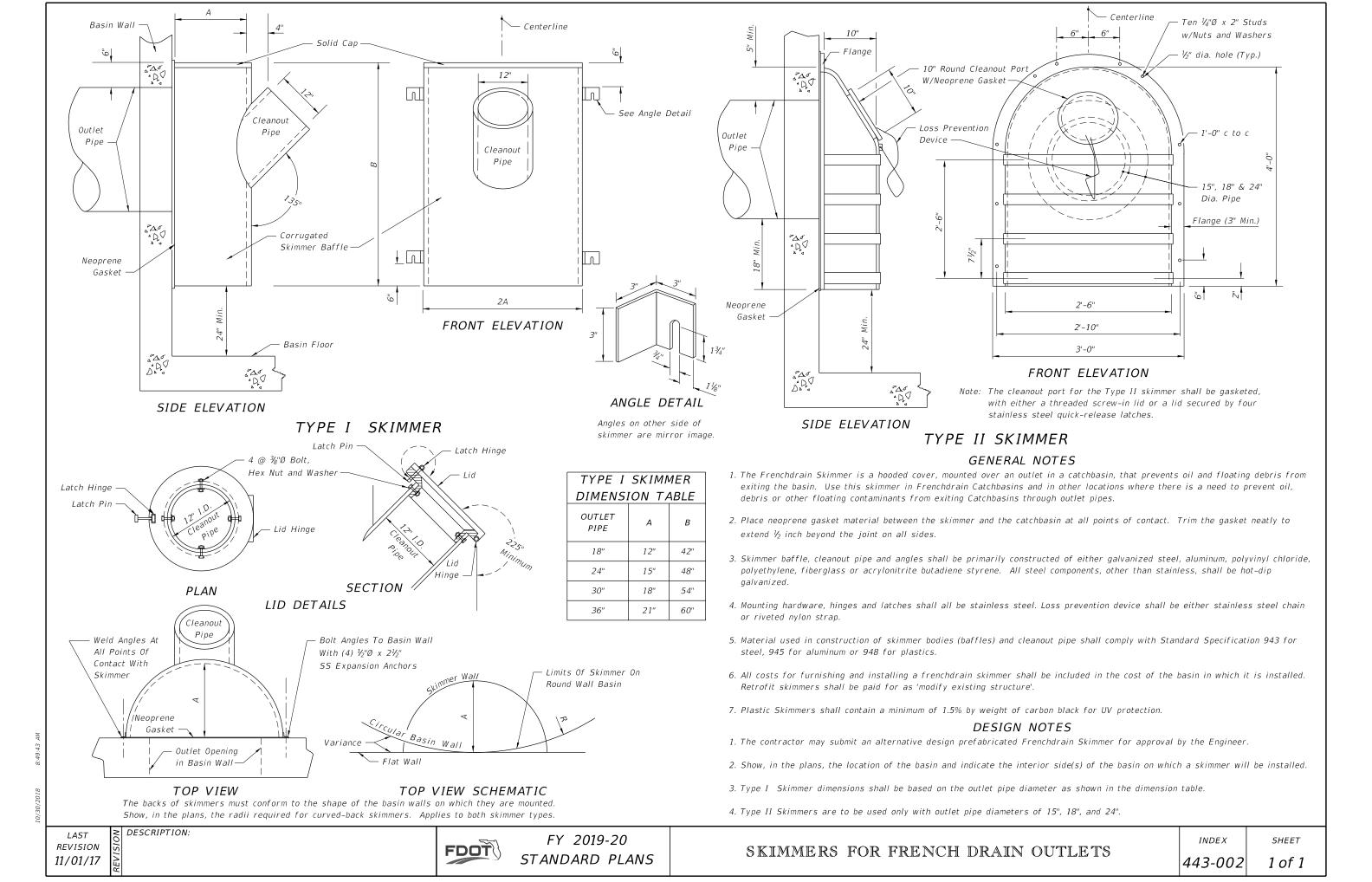
≥ DESCRIPTION:

FY 2019-20 STANDARD PLANS

FRENCH DRAIN

INDEX 443-001

SHEET 2 of 2







24" STEEL WELL GRATE

Total Opening: 1.7 sq ft minimum

For 24" well, outer diameter = 29"

DESCRIPTION:

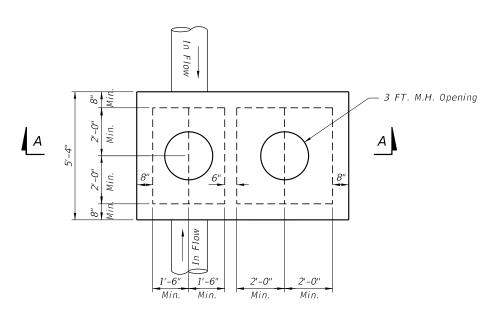
Steel well grate to be installed over 24" deep well.

Steel grate to be hot dipped galvanized after

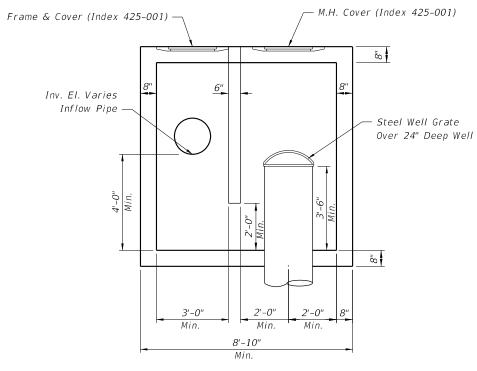
fabrication, see Specification Section 962.

Heavy duty "bee hive" grate

Openings:  $1-\frac{1}{2}$ " maximum

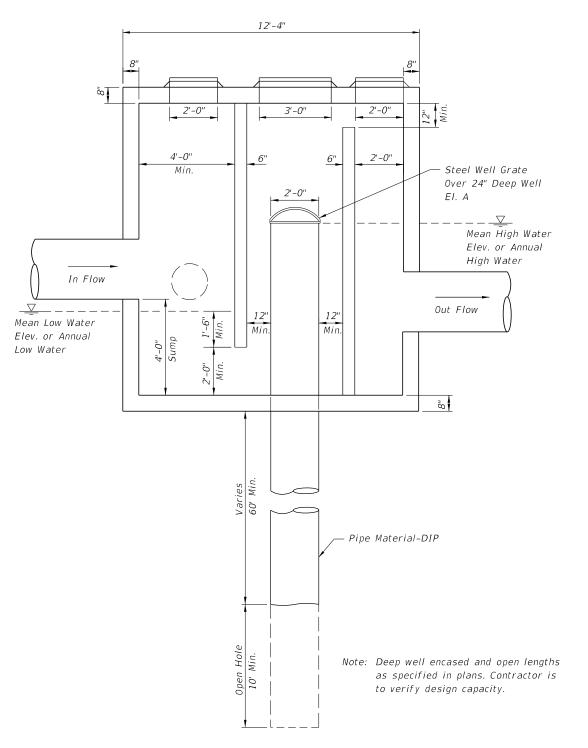


TOP SLAB PLAN



SECTION A-A

STRUCTURE WITH NO OUTFLOW



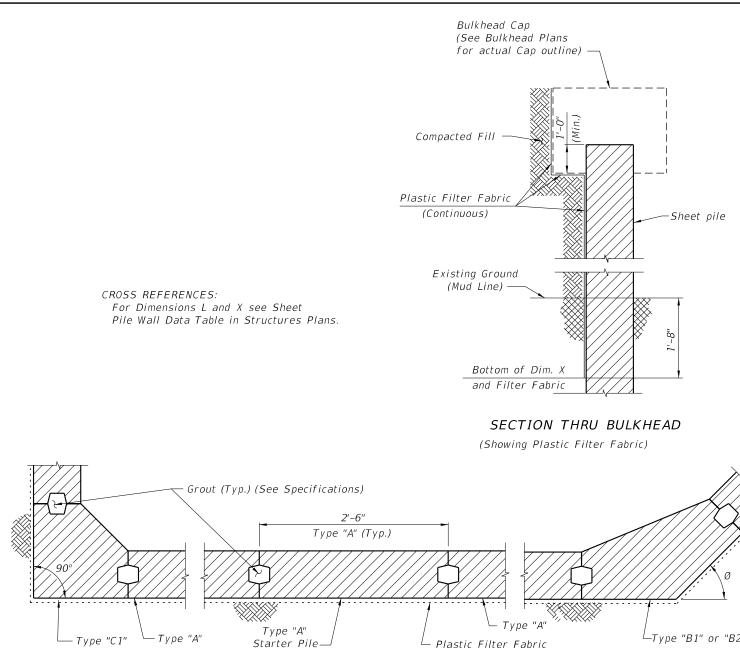
# SPECIAL MANHOLE STRUCTURE DETAIL WITH OUTFALL

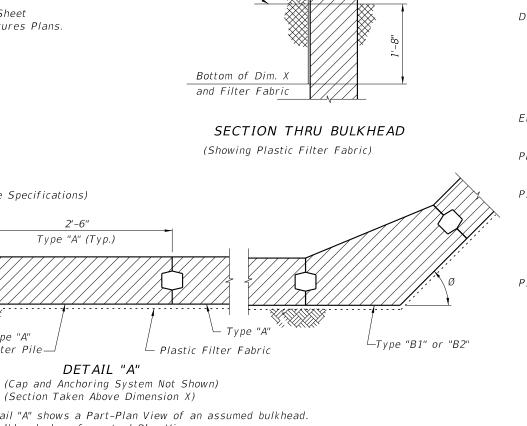
## DESIGN NOTES:

- 1. Depth of Casing Varies, 60' min.
- 2. Depth of Open Hole, 10'-20'.
- 3. Actual Size Of The Inflow And Outflow Chambers Will Be Determined By The Size Of The Pipes (Refer To Table 3 Of Index 425-010). The Width Of The Box Shall Be Constant Based On The Largest Pipe. The Length Is To Be Adjusted Based On Size and Orientation Of The Pipes.

FDOT

FY 2019-20 STANDARD PLANS





# SHEET PILE DESIGN CRITERIA AND NOTES

#### DESCRIPTION:

This Index includes details for five types of piles with two thicknesses.

Types "B1", "B2", "C1" and "C2" piles (corner piles) are of reinforced concrete construction, and Type "A" is of prestressed concrete construction. The piles shall be manufactured, cured and installed in accordance with the requirements of the contract documents.

MATERIALS: (for materials not listed refer to the Specifications)

CONCRETE

Class: V (Special) for slightly and moderately aggressive environments

V (Special) with silica fume, metakaolin or ultrafine fly ash for

extremely aggressive environments

Unit weight:

Modulus of Elasticity: Based on the use of Florida limerock concrete

REINFORCING STEEL

ASTM A615 Grade 60

#### PRESTRESSING STEEL

ASTM A416 Grade 270 (Low-Relaxation Strand)

#### DESIGN PARAMETERS:

Type "A"

Concrete Compressive Strength at release of prestressing: 4000 psi minimum Uniform compression after prestressing losses: 1000 psi minimum

Pick-up, Storage and Transportation: 0.0 psi tension with 1.5 times pile self weight

Types "B1", "B2", "C1" & "C2"

Pick-up, Storage and Transportation: Minimum compressive strength  $f'(c) \ge 4000$  psi required.

#### ENVIRONMENT:

The pile designs are applicable to all Environments.

#### PLASTIC FILTER FABRIC:

The plastic filter fabric shall extend to the bottom of the "X" dimension.

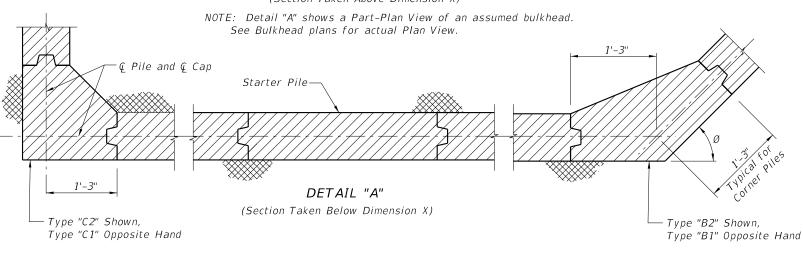
#### PILE PICK-UP AND HANDLING:

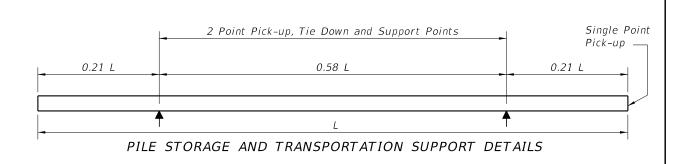
Type "A"

Pick-up of pile may be either a single point pick-up or a two point pick-up as shown below. Types "B1", "B2", "C1" & "C2"

Two point pick-up for lifting out of forms & two point support for storage & transportation. Single point pick-up for installation only.

The 2'-6" Sheet Pile dimension is nominal. This dimension may be shortened by the Manufacturer up to  $\frac{1}{2}$ " to allow for Sheet Pile fit-up in its final position. Minimum Sheet Pile width is 2'-5\\frac{1}{2}''. No changes shall be made to the tongues or grooves.





NOTES AND DETAILS

**REVISION** 11/01/17 or "C2"

DESCRIPTION:

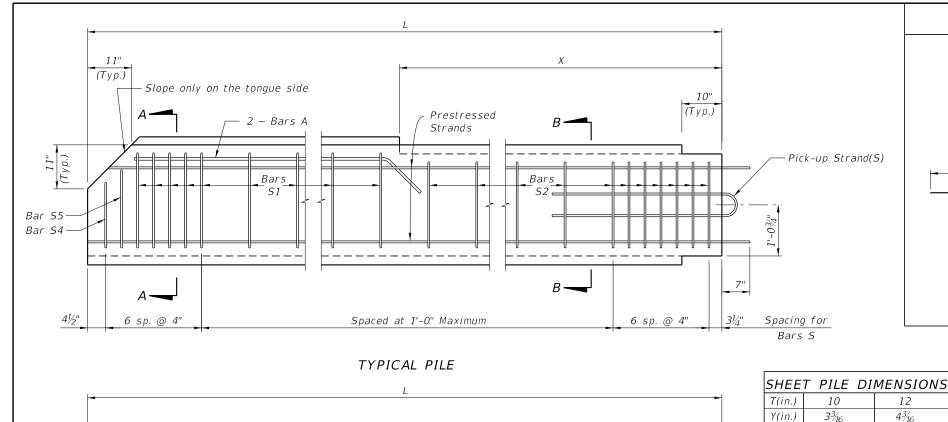
**FDOT** 

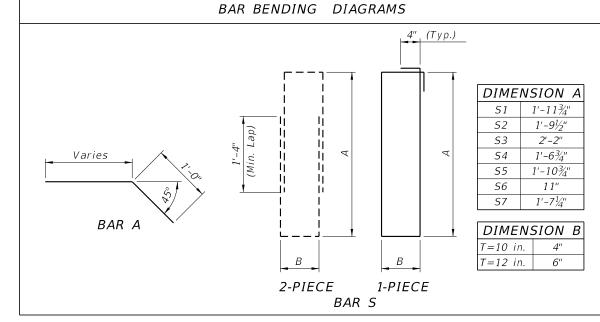
FY 2019-20 STANDARD PLANS

PRECAST CONCRETE SHEET PILE WALL (CONVENTIONAL)

INDEX 455-400

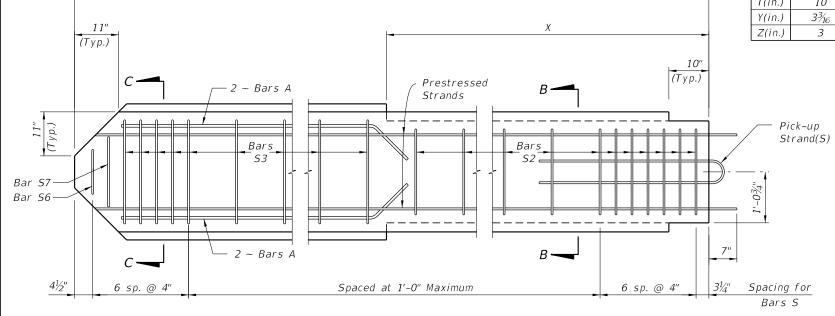
SHEET 1 of 4





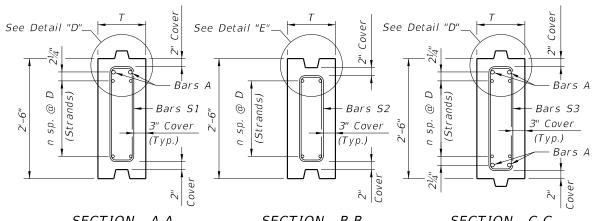
#### NOTES:

- 1. Intermediate Prestress Strands not shown in Elevations and Sections.
- All bar dimensions are out-to-out.
- 3. Bars A are #5 and Bars S are #4.
- 4. At the Contractor's option Bars S may be fabricated as a two piece bar as shown in the Bar Bending Diagram.
- 5. The Contractor may use Deformed Welded Wire Reinforcement meeting the requirements of Specification Section 931 in lieu of Bars A and Bars S if the wire size and spacing provide the same area of reinforcing steel per foot as the Bars shown.
- 6. For Dimensions L and X see Sheet Pile Data Table in Structures Plans.



# STARTER PILE

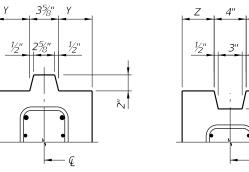
WALL THICKNESS		MAXIMUM L	n	D (in.)	TOTAL # OF STRANDS	INITIAL (JACKING) FORCE (Kip)
T=10 in.	0.5	28'-0"	6	3½	14	31
	0.6	27'-0"	4	5	10	44
T=12 in.	0.5	31'-0"	7	2%	16	31
	0.6	30'-0"	5	4	12	44



SECTION A-A

SECTION B-B

SECTION C-C



DETAIL "D" (Typical Tongue)

DETAIL "E" (Typical Groove)

TYPE "A" STANDARD SECTION

**REVISION** 11/01/18

DESCRIPTION:

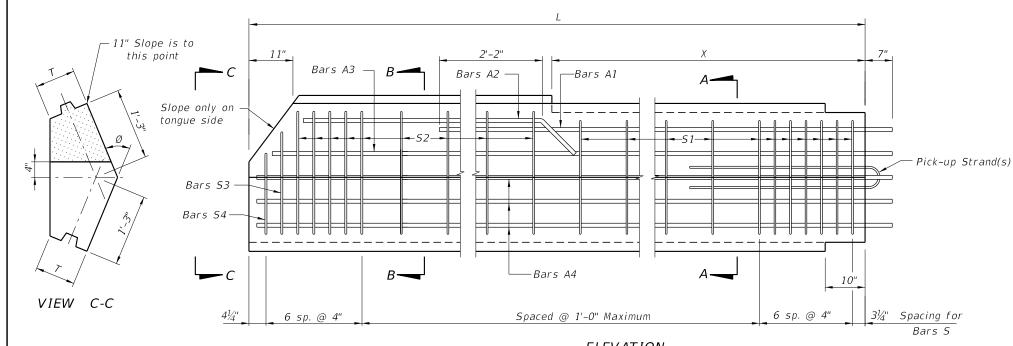
FDOT

FY 2019-20 STANDARD PLANS

PRECAST CONCRETE SHEET PILE WALL

INDEX

SHEET

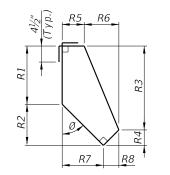


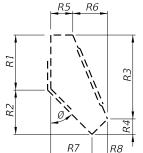
ELEVATION (TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

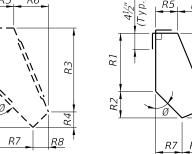
# BAR BENDING DIAGRAMS

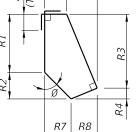
	STIRRUP DIMENSIONS (T = 10")								
Ø	BAR MARK	R1	R2	R:3	R4	R:5	R6	R7	R:8
	S1	111/4"	9¾"	1'-6½"	2½"	5"	4¾"	5½"	41/4"
30°	S-2	1'-1½"	9¾"	1'-8¾"	2½"	4½"	5½"	5¾"	41/4"
30	53	1 1½"	8"	1'-6"	11/4"	5"	4½"	4½"	5"
	54	1 1½"	41/4"	1'-13/4"	1¾"	5"	3¾"	2½"	6¼"
	S1	11½"	8"	1'-4"	4"	5½"	6½"	8"	4"
45°	52	1'-13/4"	8"	1'-5 ³ / ₄ "	4"	4½"	7½"	8"	4"
75	53	11½"	6¾"	1'-4"	21/4"	5½"	6¾"	6¾"	5½"
	54	11½"	3½"	1'-0"	3"	5½"	5"	3½"	7"
	51	1'-0"	6"	1'-03/4"	5½"	6"	7½"	10½"	3"
600	S:2	1'-2"	6"	1'-2¾"	5½"	43/4"	8¾"	10½"	3"
60°	53	1'-0"	43/4"	1'-1½"	31/4"	6"	8"	8¾"	5½"
	S4	1'-0"	2½"	10"	4½"	6"	5¾"	4"	7½"

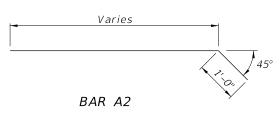
	STIRRUP DIMENSIONS (T = 12")								
Ø	BAR MARK	R1	R:2	R:3	R4	R·5	R:6	R:7	R8
	S1	11½"	10"	1'-6"	3½"	7"	4¾"	5¾"	6"
30°	S-2	1'-1¾"	10"	1'-81/4"	3½"	6½"	5½"	5¾"	6"
] 30	53	11½"	8¼"	1'-5¾"	2"	7"	4¾"	4½"	7½"
	<i>S4</i>	11½"	4"	1'-11/4"	2½"	7"	3¾"	2½"	8½"
	S1	1'-0"	8½"	1'-31/4"	5½"	7½"	$6\frac{1}{4}$ "	8½"	5½"
45°	52	1'-2½"	8½"	1'-5½"	5½"	6½"	7½"	8½"	5½"
45	53	1'-0"	7"	1'-4"	3"	7½"	6¾"	7"	7½"
	54	1'-0"	3½"	1 1 ³ / ₄ "	3¾"	7½"	5"	3½"	9"
	S1	1'-01/2"	$6\frac{1}{4}''$	1 1 ¾"	7"	8"	6¾"	10¾"	4"
60°	52	1'-2 ³ / ₄ ''	$6\frac{1}{4}''$	1'-2"	7"	6¾"	8"	10¾"	4"
	<i>S3</i>	1'-0½"	5"	1'-1½"	4"	8"	8"	9"	7"
	54	1'-01/2"	2½"	9½"	5½"	8"	5½"	41/4"	91/4"











1 - PIECE 2 - PIECE BARS S1 & S2

DESCRIPTION:

BARS S3 & S4



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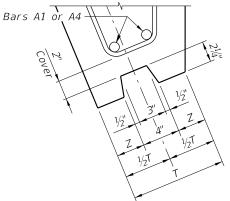


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See Detail "D" Bars A2 Bars A1 ²Bars A2 Bars A3 Bars A3 Bars A3 Bars A4 Bars A4 Bars A4 Bars S2 Bars A4 Bars A4 Bars A4 Bars A4 3" Cover (Typ.)See Detail "D" 3" Cover Typ.SECTION B-B SHEET PILE DIMENSIONS T (in.)  $3\frac{3}{16}$  $4\frac{3}{16}$ Y (in.) Z (in.) Bars S



SECTION A-A

DETAIL "D" (TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

- 1. This drawing includes details for precast concrete corner piles for 10" and 12" thick sheet pile systems. The details apply equally to both thicknesses.
- 2. The bar configurations shown in Sections A-A and B-B shall be used for Ø angles between 15° and 75°. For Ø angles not shown, the reinforcing bar dimensions may be interpolated or extrapolated from the stirrup dimensions shown.
- 3. All bar dimensions are out-to-out.

Bars A3

Bars S1

Bars A4

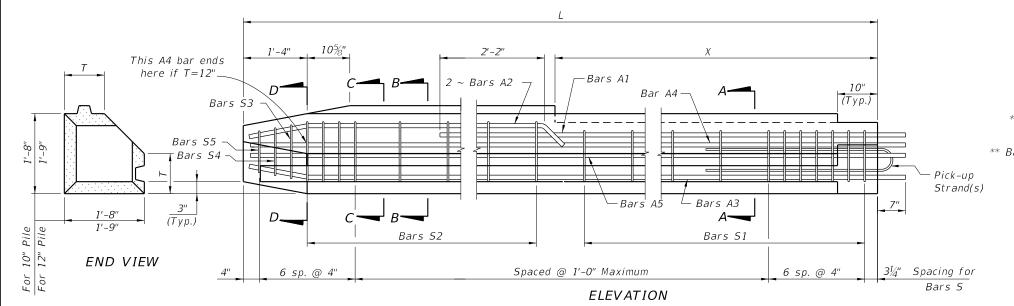
Bars A4

- 4. Bars A are #8 and Bars S are #4.
- 5. Values for Stirrup Dimensions are shown for Ø equal to 30°, 45° & 60° only.
- 6. At the Contractor's option Bars S may be fabricated as a 2 piece bar with a minimum lap length of 1'-4", as shown in Bar Bending Diagrams.
- 7. If Type "B1" or "B2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3 & S4 in shop drawings.
- 8. If tongue must be on the opposite side from that shown all dimensions and Bars A,
- S2, S3 and S4 will be the same but opposite hand. 9. For Dimensions L, X and Angle Ø, see Sheet Pile Data Table in Structures Plans.

TYPE "B1" AND "B2" - VARIABLE ANGLE CORNER PILE

**REVISION** 11/01/16

FDOT

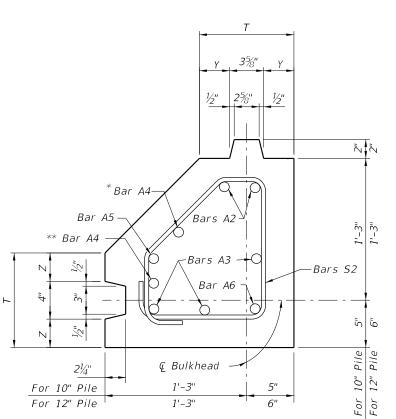


(TYPE "C1" PILE SHOWN, TYPE "C2" PILE OPPOSITE HAND)

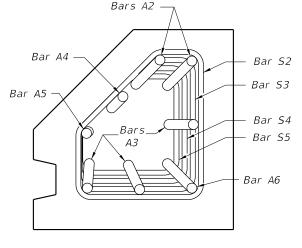
Pile Pile Section C-C 3" Cover Bars A1 * Bar A4 Bar A5 Bar A5 Bars A2 ** Bar A4 ** Bar A4 2" (Typ.) - 3" Cover See Section C-C For 10" Pile For 12" Pile SECTION A-A SECTION B-B

* This Bar A4 shall be 1'-2'' shorter than other A4 bars for T = 12''.

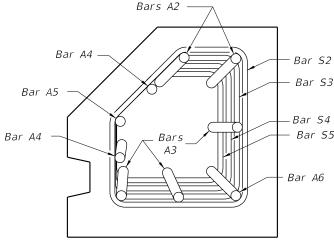
** This Bar A4 (not shown in elevation) is included only if T = 12".



SECTION C-C (T=10" or 12")



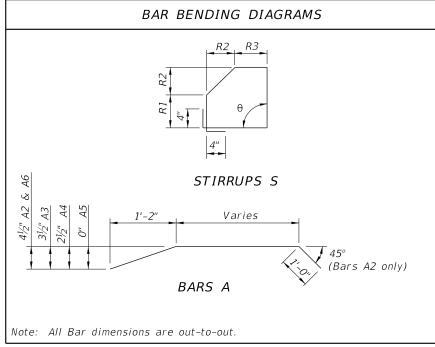
SECTION D-D (T=10")



SECTION D-D (T=12")

	STIRRUP DIMENSIONS					
θ	T (in.)	BAR MARK	R1	R:2	R:3	
		51	7"	5¾"	7"	
		52	7"	8"	4¾"	
	10	53	6¼"	7½"	4¾"	
		54	5½"	6½"	4¾"	
90°		<i>S5</i>	$4\frac{3}{4}$ "	5¾"	4¾"	
90		S1	9"	43/4"	9"	
		52	9"	7"	6¾"	
	12	53	8½"	$6\frac{1}{4}$ "	6¾"	
		54	7½"	5½"	6¾"	
		<i>S5</i>	6¾"	4¾"	6¾"	

#### SHEET PILE DIMENSIONS T (in.) 10 12 $3\frac{3}{16}$ Y (in.) $4\frac{3}{16}$ 3 Z (in.)



#### NOTES:

- 1. All bar dimensions are out-to-out.
- 2. Bars A are #8 and Bars S are #4.
- 3. This drawing includes information for precast Corner Piles for 10" and 12" thick Sheet Pile systems. The details apply to both thicknesses but the bar configurations change slightly according to the thickness values used.
- 4. If Type "C1" or "C2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3, S4 & S5 in shop drawings.
- 5. If tongue must be on opposite side (Groove Side) from that shown, all dimensions and reinforcement shall follow the corresponding Tongue
- 6. For Dimensions L and X see Sheet Pile Data Table in Structures Plans.

TYPE "C1" AND "C2" - RIGHT ANGLE CORNER PILE

DESCRIPTION: **REVISION** 07/01/12

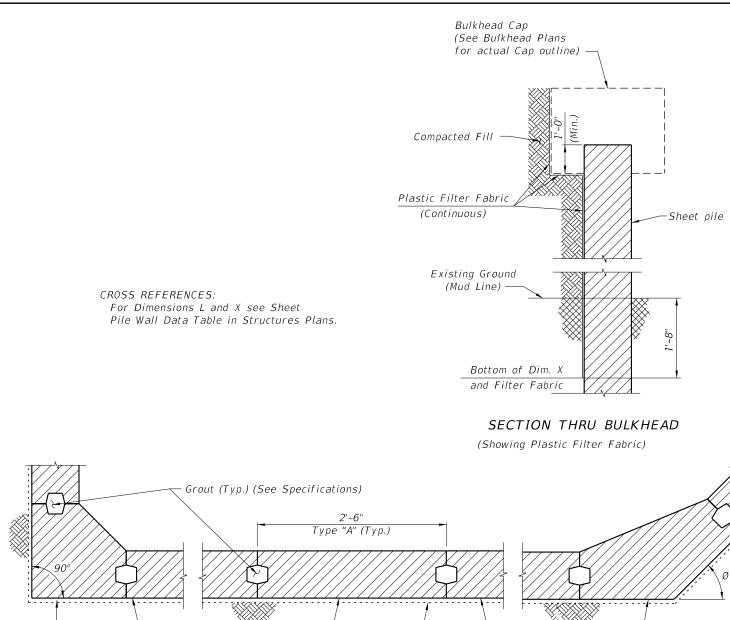
FDOT

FY 2019-20 STANDARD PLANS

PRECAST CONCRETE SHEET PILE WALL

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Type "A"

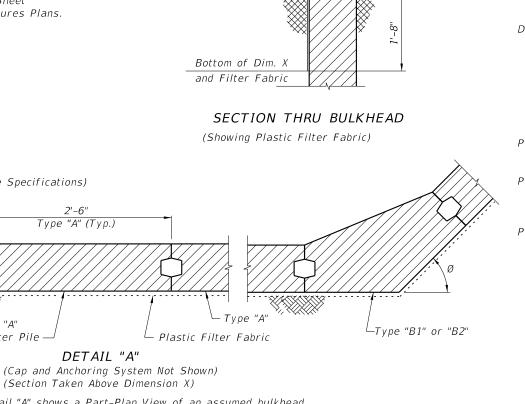
Starter Pile -

Type "A"

- Type "C1"

DESCRIPTION:

or "C2"



# CFRP/GFRP SHEET PILE DESIGN CRITERIA AND NOTES

#### DESCRIPTION:

This Index includes details for six types of piles with two thicknesses. Type "A" is prestressed concrete construction with CFRP or HSSS strands. Types "B1", "B2", "C1" and "C2" piles (corner piles) are reinforced concrete construction. Manufacture, cure and install Sheet Piles in accordance with the requirements of the contract documents.

MATERIALS: (for materials not listed refer to the Specifications)

#### CONCRETE

V (Special)

Class: Unit weight: 145 pcf

Modulus of Elasticity: Based on the use of Florida limerock aggregate concrete

#### REINFORCING BARS

Glass Fiber Reinforced Polymer (GFRP) bars meeting the requirements of Specification Section 932.

#### PRESTRESSING STRAND

Stainless Steel: Prestressing steel shall be seven-wire HSSS, UNS 532205 (Type 2205) or UNS S31803 strand, meeting the requirements of Specification Section 933.

Carbon FRP: Prestressing strand shall be CFRP strand, meeting the requirements of Specification Section 933.

#### DESIGN PARAMETERS:

#### Type "A"

Concrete Compressive Strength at release of prestressing:

Uniform compression after prestressing losses:

Pick-up, Storage and Transportation:

700 psi minimum 450 psi tension with 1.5 times pile self weight for single-point pick-up at f'c ≥ 6000 psi

4000 psi minimum

## Types "B1", "B2", "C1" & "C2"

Pick-up, Storage and Transportation: Minimum compressive strength f'ci ≥ 4000 psi required for two-point pick-up;  $f'c \ge 6000$  psi for single-point pick-up.

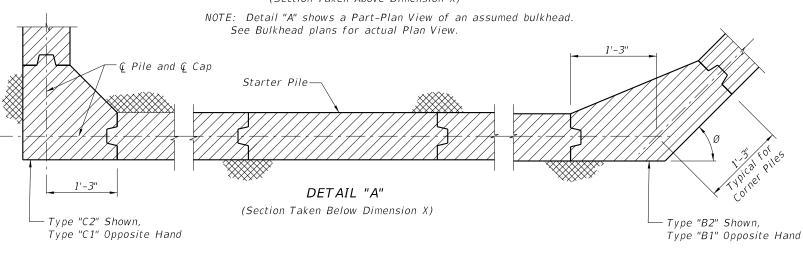
#### PLASTIC FILTER FABRIC:

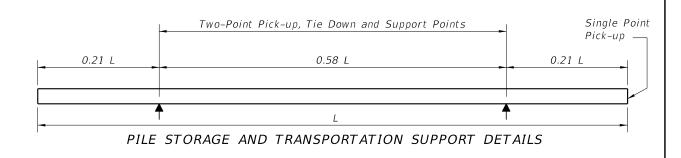
The plastic filter fabric shall extend to the bottom of the "X" dimension.

#### PILE PICK-UP AND HANDLING:

Two-point pick-up for lifting out of forms & two-point support for storage & transportation. Single-point pick-up for installation only.

The 2'-6" Sheet Pile dimension is nominal. This dimension may be shortened by the Manufacturer up to  $\frac{1}{2}$ " to allow for Sheet Pile fit-up in its final position. Minimum Sheet Pile width is 2'-5\\'/. No changes shall be made to the tongues or grooves.





NOTES AND DETAILS

**REVISION** 11/01/16

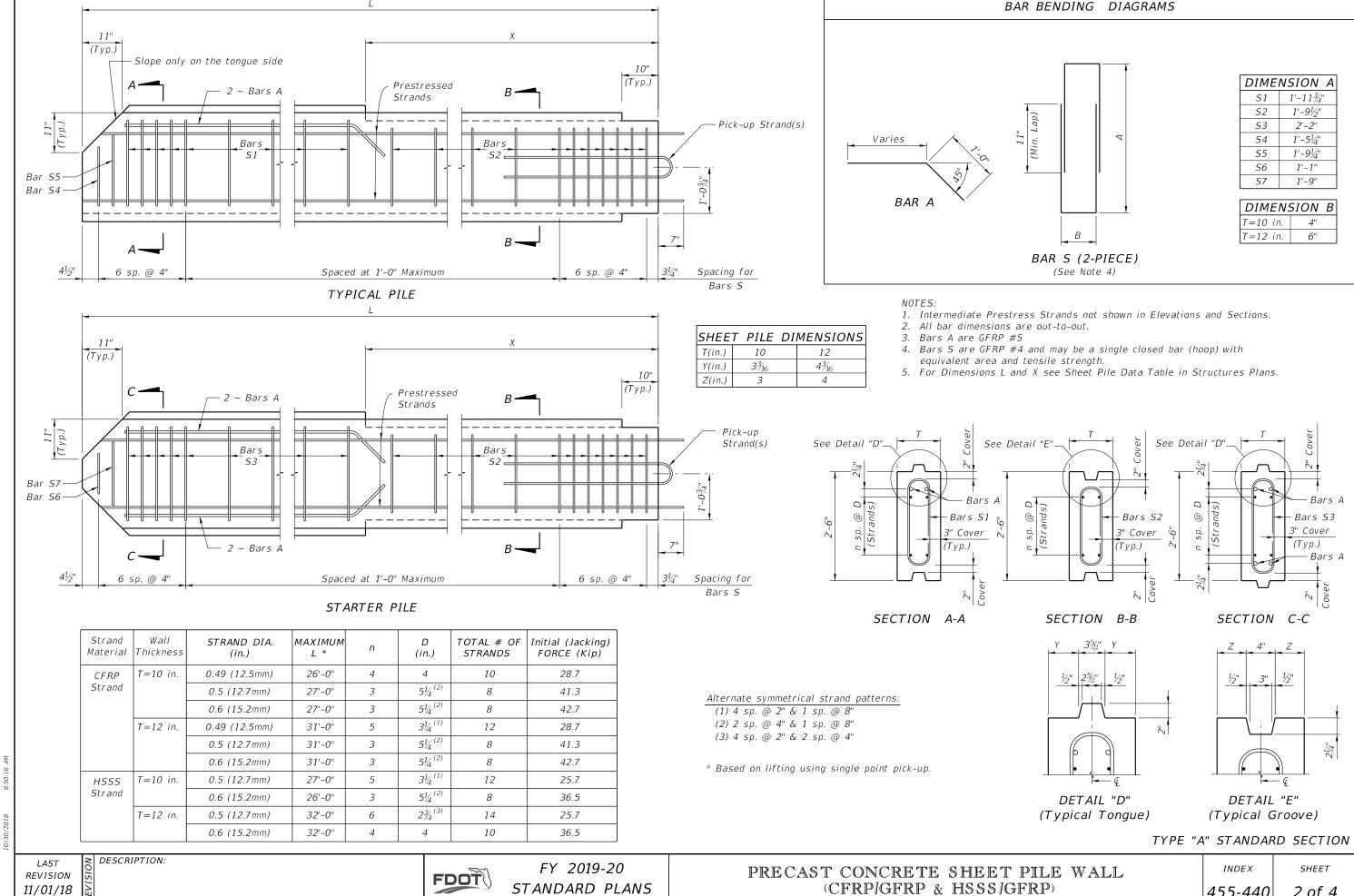
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FY 2019-20 STANDARD PLANS

PRECAST CONCRETE SHEET PILE WALL (CFRP/GFRP & HSSS/GFRP)

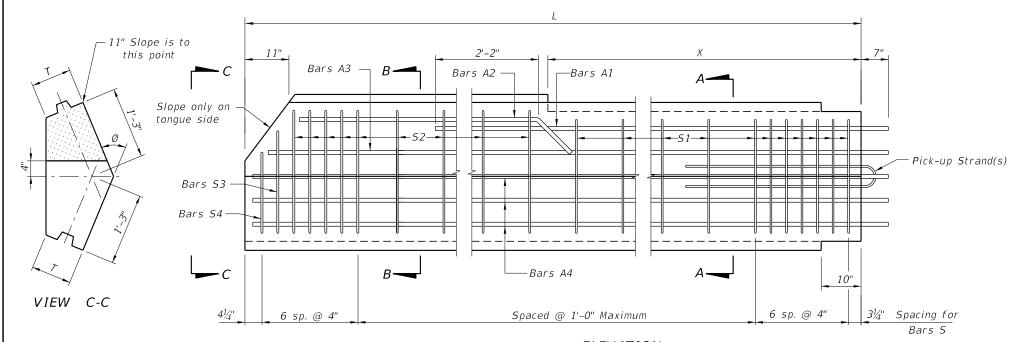
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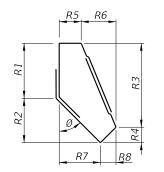


ELEVATION (TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

# BAR BENDING DIAGRAMS

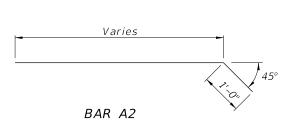
		STIF	RRUP I	DIMENS	SIONS	(T =	10")		
Ø	BAR MARK	R1	R2	R:3	R:4	R:5	R:6	R:7	R8
	<i>S1</i>	111/4"	9¾"	1'-6½"	2½"	5"	43/4"	5½"	41/4"
30°	S:2	1'-1½"	9¾"	1'-8¾"	2½"	4½"	5½"	5¾"	41/4"
30	<i>S3</i>	111/4"	8"	1'-6"	11/4"	5"	4½"	4½"	5"
	54	111/4"	41/4"	1'-13/4"	1¾"	5"	33/4"	2½"	61/4"
	S1	11½"	8"	1'-4"	4"	5½"	6½"	8"	4"
4.5°	S-2	1'-13/4"	8"	1'-5¾"	4"	4½"	7½"	8"	4"
43	53	11½"	6¾"	1'-4"	21/4"	5½"	6¾"	6¾"	5½"
	<i>S4</i>	111½"	3½"	1'-0"	3"	5½"	5"	3½"	7"
	<i>S1</i>	1'-0"	6"	1'-03/4"	5½"	6"	71/4"	101/4"	3"
C 00	<i>S</i> :2	1'-2"	6"	1'-2¾"	5½"	43/4"	8¾"	10½"	3"
60°	53	1'-0"	43/4"	1'-1½"	31/4"	6"	8"	8¾"	5½"
	C 1	11 011	21/11	1.0"	A1/_"	CII	E 3/11	1"	71/"

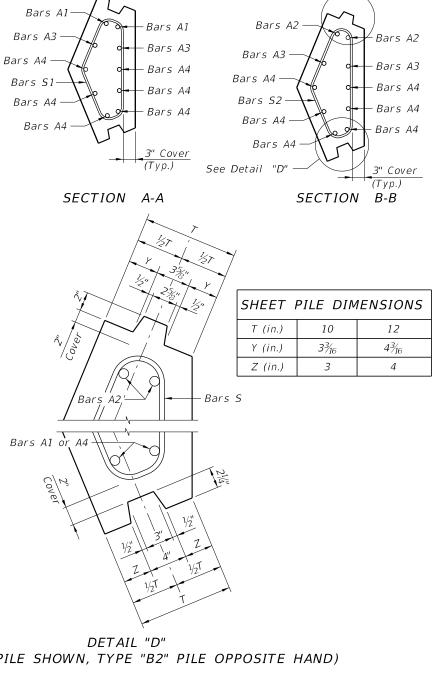
	STIRRUP DIMENSIONS (T = 12")								
Ø	BAR MARK	R1	R2	R:3	R4	R:5	R6	R7	R8
	S1	11½"	10"	1'-6"	3½"	7"	4¾"	5¾"	6"
30°	S2	1'-13/4"	10"	1'-81/4"	3½"	6½"	5½"	5¾"	6"
30	<i>S3</i>	11½"	8¼"	1'-5¾"	2"	7"	4¾"	4½"	71/4"
	<i>S4</i>	11½"	4"	1'-11/4"	2½"	7"	3¾"	2½"	8¼"
	<i>S1</i>	1'-0"	8½"	1'-31/4"	5½"	7½"	$6\frac{1}{4}$ "	8½"	5½"
45°	<i>S2</i>	1'-21/4"	8½"	1'-5½"	5½"	6½"	7½"	8½"	5½"
45	53	1'-0"	7"	1'-4"	3"	7½"	6¾"	7"	71/4"
	<i>S4</i>	1'-0"	3½"	11¾"	3¾"	7½"	5"	3½"	9"
	S1	1'-01/2"	$6\frac{1}{4}''$	11¾"	7"	8"	6¾"	10¾"	4"
60°	52	1'-2¾''	$6\frac{1}{4}''$	1'-2"	7"	6¾"	8"	10¾"	4"
	53	1'-0½"	5"	1'-1½"	4"	8"	8"	9"	7"
	54	1'-01/2"	21/2"	9½"	5½"	8"	5½"	41/4"	91/4"



BARS S1 & S2 (2 - PIECE)

DESCRIPTION:





See Detail "D"

(TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

- 1. This drawing includes details for precast concrete corner piles for 10" and 12" thick sheet pile systems. The details apply equally to both thicknesses.
- 2. The bar configurations shown in Sections A-A and B-B shall be used for Ø angles between 15° and 75°. For Ø angles not shown, the reinforcing bar dimensions may be interpolated or extrapolated from the stirrup dimensions shown.
- All bar dimensions are out-to-out.
- Bars A are GFRP #8 and Bars S are GFRP #4.
- 5. Values for Stirrup Dimensions are shown for  $\emptyset$  equal to  $30^\circ$ ,  $45^\circ$  &  $60^\circ$  only.
- 6. Bars S are fabricated as a 2 piece stirrup with a minimum lap length of 8", as shown in Bar Bending Diagrams, or a single closed bar (hoop) when approved by the Engineer.
- 7. If Type "B1" or "B2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3 & S4 in shop drawings.
- 8. If tongue must be on the opposite side from that shown all dimensions and Bars A, S2, S3 and S4 will be the same but opposite hand.
- 9. For Dimensions L, X and Angle Ø, see Sheet Pile Data Table in Structures Plans.

TYPE "B1" AND "B2" - VARIABLE ANGLE CORNER PILE

**REVISION** 11/01/16

FDOT

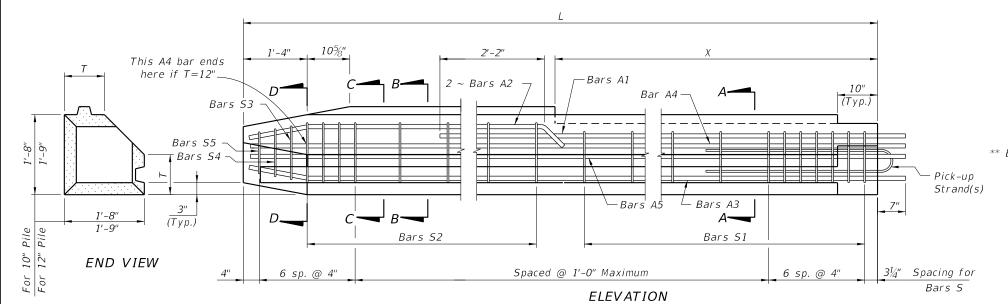
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(CFRP/GFRP & HSSS/GFRP)

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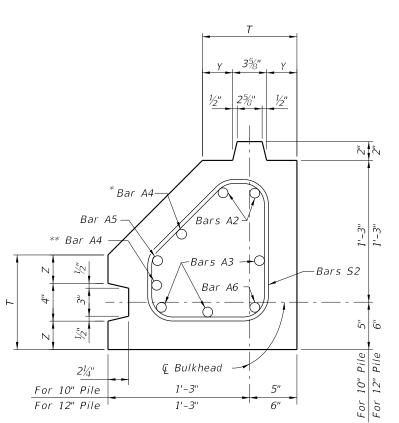


(TYPE "C1" PILE SHOWN, TYPE "C2" PILE OPPOSITE HAND)

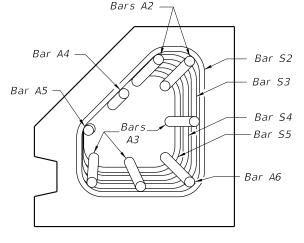
Pile Pile Section C-C 3" Cover Bars A1 * Bar A4 Bar A5 Bar A5 Bars A2 ** Bar A4 ** Bar A4 2" (Typ.) - 3" Cover See Section C-C For 10" Pile For 12" Pile SECTION A-A SECTION B-B

* This Bar A4 shall be 1'-2'' shorter than other A4 bars for T = 12''.

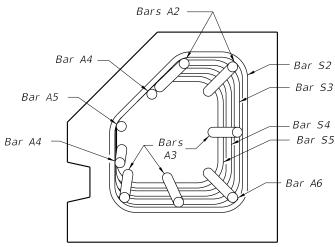
** This Bar A4 (not shown in elevation) is included only if T = 12".



SECTION C-C (T=10" or 12")



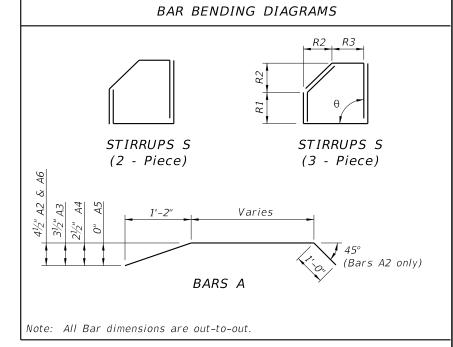
SECTION D-D (T=10")



SECTION D-D (T=12")

	STIRRUP DIMENSIONS					
θ	T (in.)	BAR MARK	R1	R·2	R:3	
		51	7"	5¾"	7"	
		52	7"	8"	4¾"	
	10	53	6¼"	71/4"	4¾"	
		54	5½"	6½"	4¾"	
90°		<i>S5</i>	$4\frac{3}{4}$ "	5¾"	4¾"	
90		S1	9"	43/4"	9"	
		52	9"	7"	6¾"	
	12	53	8½"	$6\frac{1}{4}$ "	6¾"	
		54	7½"	5½"	6¾"	
		<i>S5</i>	6¾"	43/4"	6¾"	

#### SHEET PILE DIMENSIONS T (in.) 10 12 $3\frac{3}{16}$ Y (in.) $4\frac{3}{16}$ 3 Z (in.)



#### NOTES:

- 1. All bar dimensions are out-to-out.
- 2. Bars A are GFRP #8 and Bars S are GFRP #4.
- 3. This drawing includes information for precast Corner Piles for 10" and 12" thick Sheet Pile systems. The details apply to both thicknesses but the bar configurations change slightly according to the thickness values used.
- 4. If Type "C1" or "C2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3, S4 & S5 in
- 5. At the Contractor's option Bars S may be fabricated as a 2 piece or 3 piece bar with a minimum lap length of 8", as shown in Bar Bending Diagrams, or as a single closed bar (hoop) when approved by the Engineer.
- 6. If tongue must be on opposite side (Groove Side) from that shown, all dimensions and reinforcement shall follow the corresponding Tongue or Groove side.
- 7. For Dimensions L and X see Sheet Pile Data Table in Structures Plans.

TYPE "C1" AND "C2" - RIGHT ANGLE CORNER PILE

**REVISION** 11/01/16

DESCRIPTION:

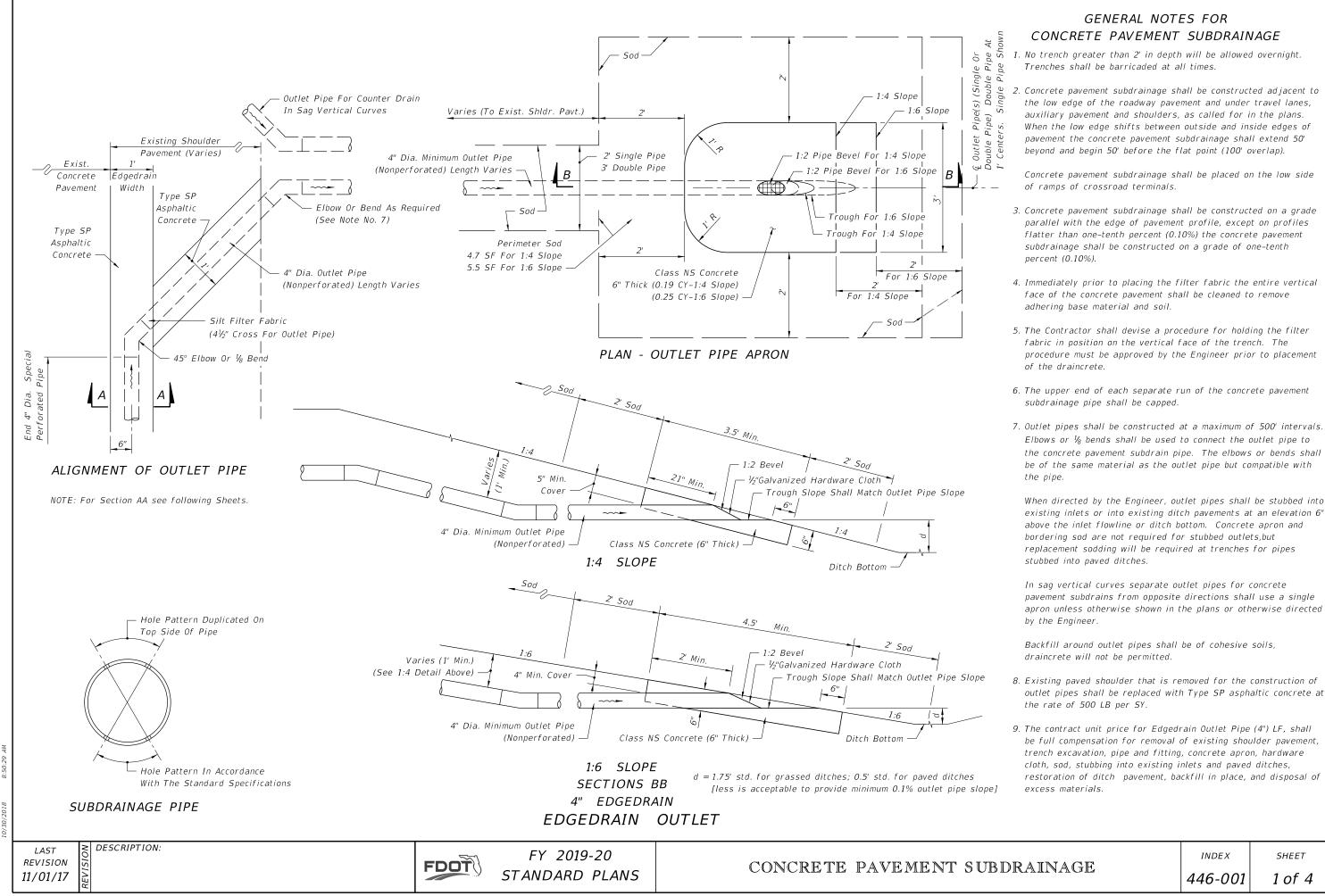
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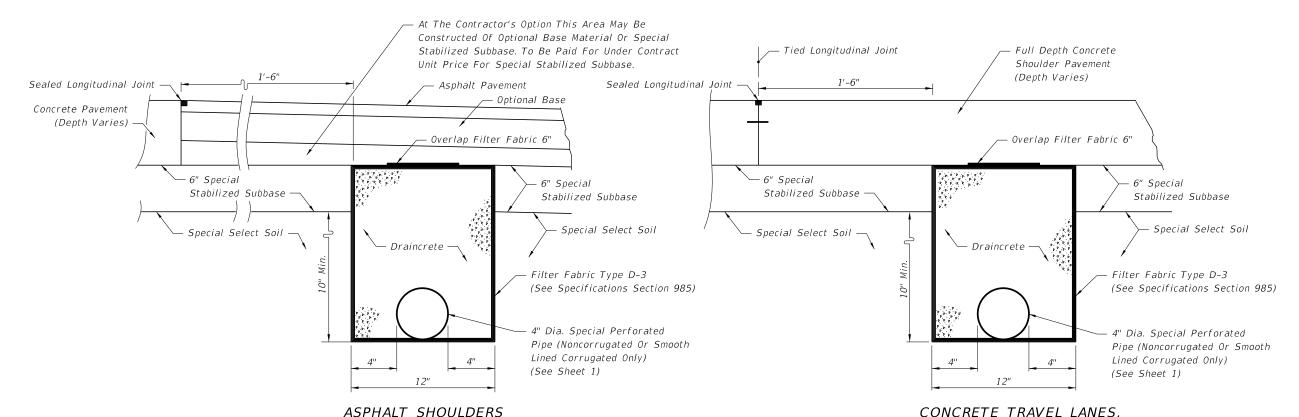
FY 2019-20 STANDARD PLANS PRECAST CONCRETE SHEET PILE WALL

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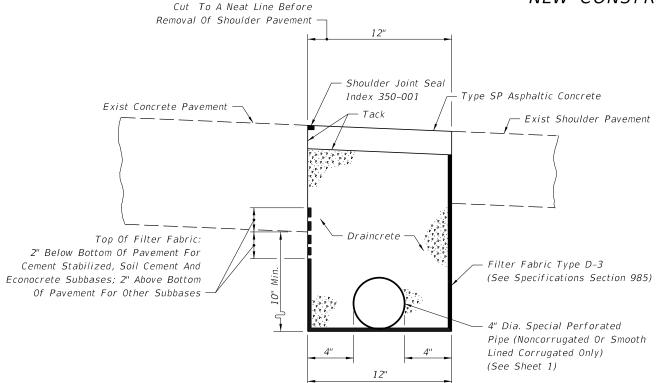
(CFRP/GFRP & HSSS/GFRP)





# CONCRETE TRAVEL LANES, SHOULDERS, AND AUXILIARY PAVEMENT

# NEW CONSTRUCTION



# DRAINCRETE SUBDRAINAGE

REHABILITATION

# NOTES FOR DRAINCRETE PAVEMENT SUBDRAINAGE

- 1. The edgedrain sections for DRAINCRETE SUBDRAINAGE are applicable to pavement construction identified as RIGID PAVEMENT on Index 120-001.
- 2. The contractor shall confine the construction of draincrete edgedrain to an area in which the entire operation can be carried out in five (5) work days, unless another construction period is called for in the plans, with sufficient time allowed for the draincrete to set before placement of pavement.

# METHOD OF PAYMENT

## **NEW CONSTRUCTION:**

1. The contract unit price for Edgedrain (Draincrete) LF shall be full compensation for trench excavation, disposal of excess material, filter fabric, draincrete edgedrain pipe and fittings and draincrete.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.

#### FOR REHABILITATION:

1. The contract unit price for Edgedrain (Draincrete) LF, shall be full compensation for removal of existing shoulder pavement, trench excavation, disposal of excess materials, filter fabric, draincrete edgedrain pipe and fittings, and draincrete, necessary for edgedrain construction.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.

Shoulder pavement shall be paid for under the contract unit price for Type SP, Asphaltic Concrete.

Shoulder joint seal shall be paid for under the contract unit price for Pavement Joint, LF.

**REVISION** 

FDOT

FY 2019-20 STANDARD PLANS

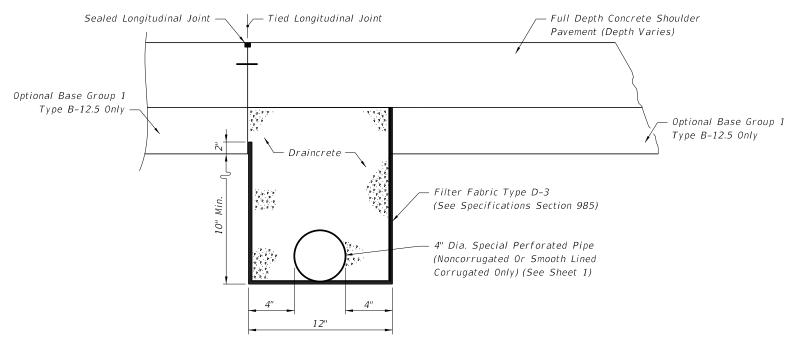
CONCRETE PAVEMENT SUBDRAINAGE

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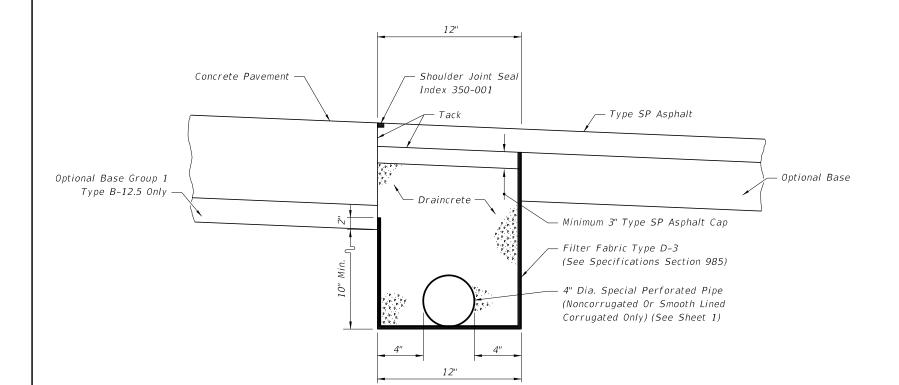
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11/01/17



CONCRETE TRAVEL LANES, SHOULDERS, AND AUXILIARY PAVEMENT



# ASPHALT SHOULDERS

# ASPHALT BASE SUBDRAINAGE

# NOTES FOR DRAINCRETE PAVEMENT SUBDRAINAGE

- 1. The edgedrain sections for DRAINCRETE SUBDRAINAGE are applicable to pavement construction identified as RIGID PAVEMENT on Index 120-001.
- 2. The contractor shall confine the construction of draincrete edgedrain to an area in which the entire operation can be carried out in five (5) work days, unless another construction period is called for in the plans, with sufficient time allowed for the draincrete to set before placement of pavement.

## METHOD OF PAYMENT

#### **NEW CONSTRUCTION:**

1. The contract unit price for Edgedrain (Draincrete) LF shall be full compensation for trench excavation, disposal of excess material, filter fabric, draincrete edgedrain pipe and fittings and draincrete.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.

- 2. Type B-12.5 shall be paid for under the contract unit price for Optional Base.
- 3. Shoulder pavement shall be paid for under the contract unit price for Type SP, Asphaltic Concrete.

**REVISION** 11/01/17

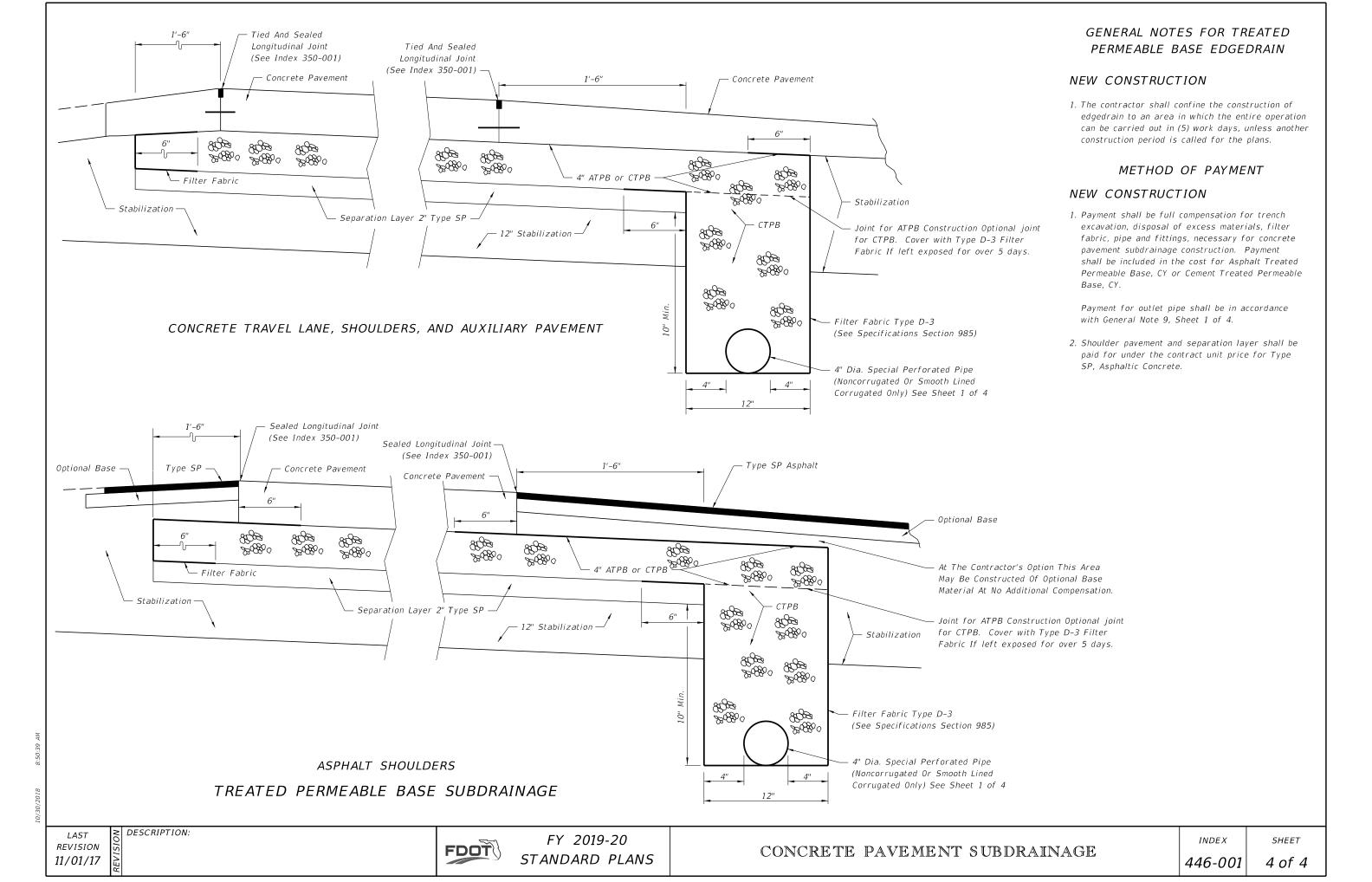
DESCRIPTION:

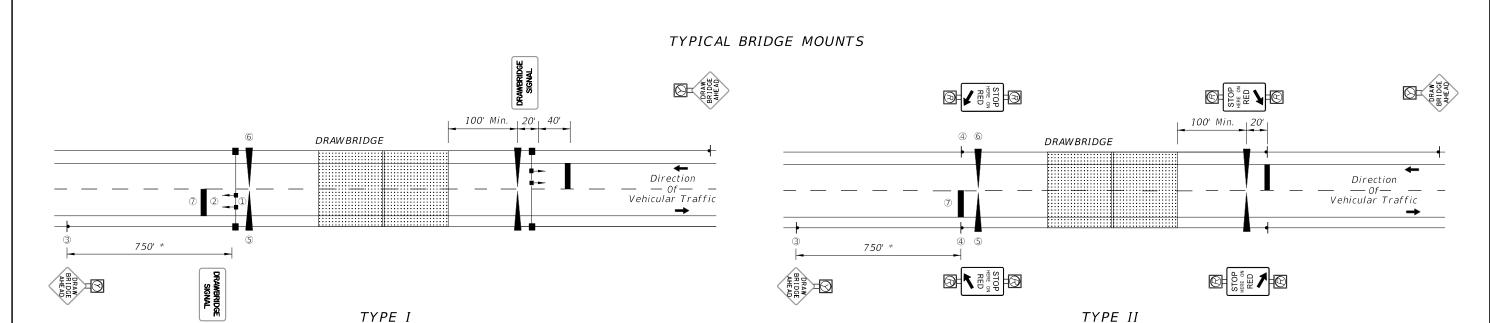
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TO BE USED WHERE BRIDGE OPERATORS ARE FULL TIME OR A DAILY BASIS.

TO BE USED WHERE TYPE I IS NOT APPLICABLE (USUALLY WHEN THE BRIDGE OPERATOR IS "ON CALL").

#### LEGEND:

- ① TRAFFIC SIGNALS ) Mast Arm Mounted (Off Bridge)
- ② DRAWBRIDGE SIGN J Monotube Support Mounted (On Bridge)
- DRAWBRIDGE AHEAD SIGN WITH YELLOW FLASHING BEACON
- STOP HERE ON RED SIGN WITH RED FLASHING BEACONS
- ENTRANCE GATE
- EXIT GATE
- 24" THERMOPLASTIC STOP BAR



SLIPPERY WHEN WET SIGN See Note 11

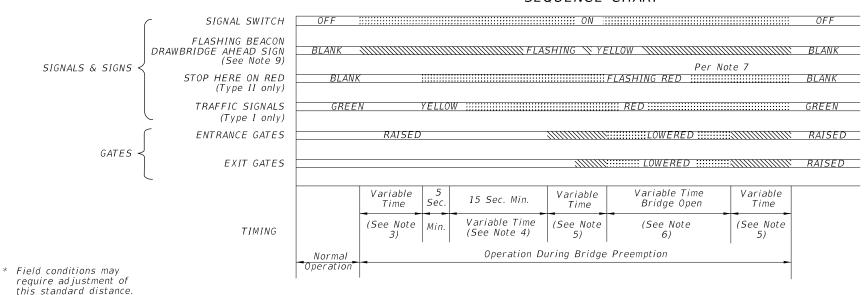
# NOTES:

- 1. A bypass switch shall be installed to override each timing interval in case of a malfunction
- 2. "STOP HERE ON RED" is omitted in Type I operation and "TRAFFIC SIGNALS" are omitted in Type II operation.
- 3. The time between beginning of flashing yellow on "Drawbridge Ahead" sign and the clearance of traffic signal to red, or beginning of flashing red should not be less than the travel time of a passenger car, from the sign location to the stop line, traveling at the 85 percentile
- 4. Beginning of operation of drawbridge gates shall not be less than 15 seconds after steady red or 20 seconds after flashing red (Actual time may be determined by the bridge tender.)
- 5. Time of gate lowering and raising is dependent upon gate type.
- 6. Time of bridge opening is determined by the bridge tender
- 7. Each gate shall be operated by a separate switch.

DESCRIPTION:

- 8. On each approach (Type II ), all four red signals shall be on the same two circuit flashers, with the two top signals on one circuit, and the two bottom signals on the alternately flashing
- 9. A Drawbridge Ahead sign is required for both types of signal operation, However a flashing beacon shall be added to the sign when physical conditions prevent a driver traveling at the 85% approach speed from having continuous view of at least one signal indication for approximately 10 seconds.
- 10. Requirements on gate installation are contained in Section 4I of the "Manual on Uniform Traffic Control Devices".
- 11. "In accordance with Traffic Engineering Manual (Topic Number 750-000-005) Section 2.1 SLIPPERY WHEN WET SIGNS shall be placed in advance of all MOVABLE and NONMOVABLE STEEL DECK BRIDGES."

# SEQUENCE CHART



Ground Mounted

LAST **REVISION** 11/01/17

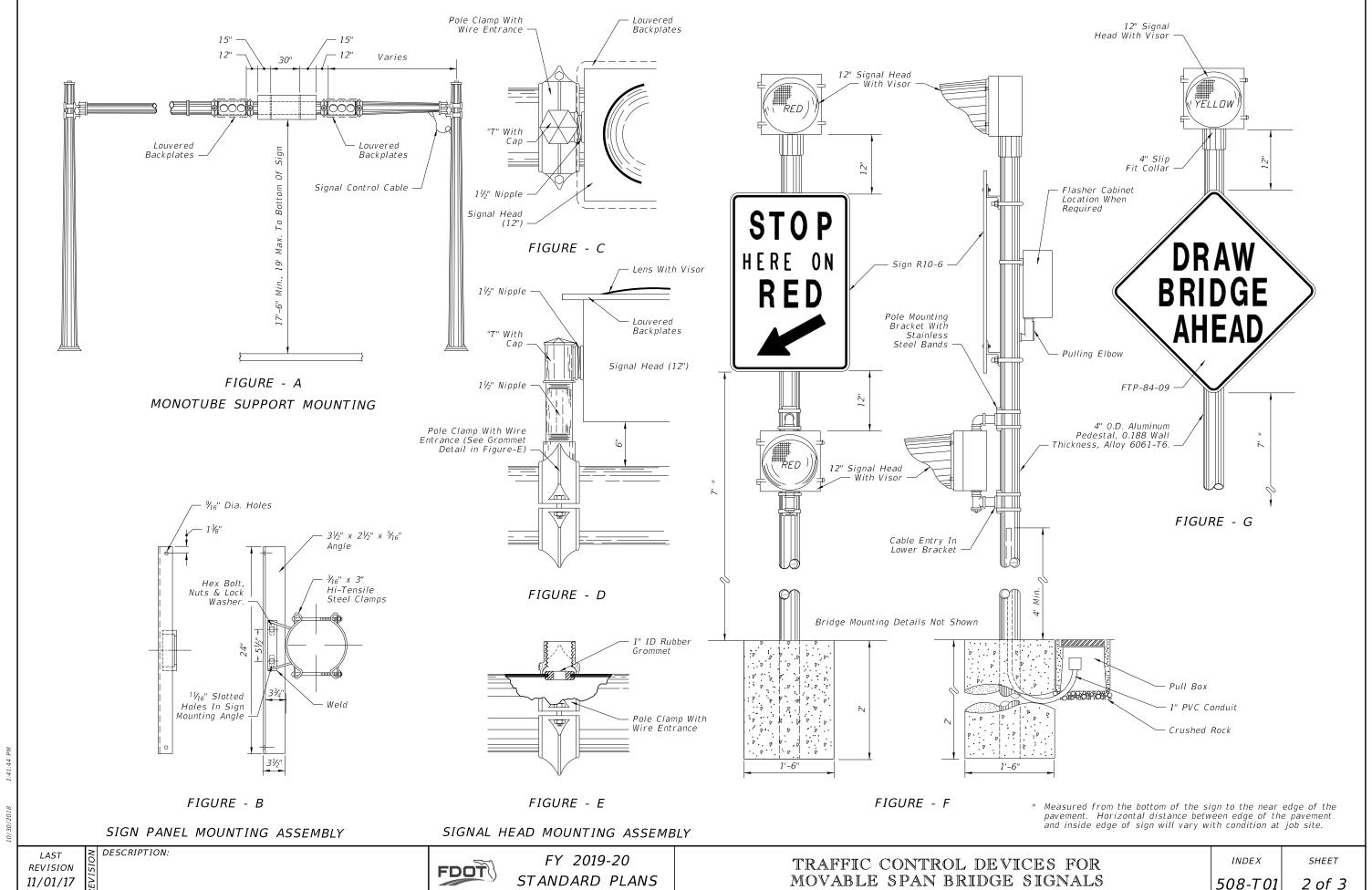
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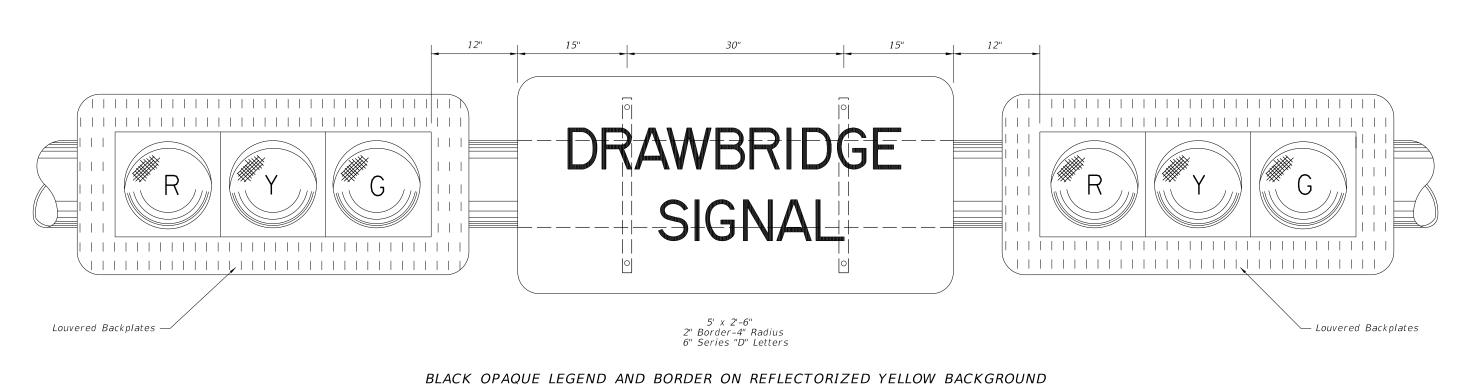
TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS

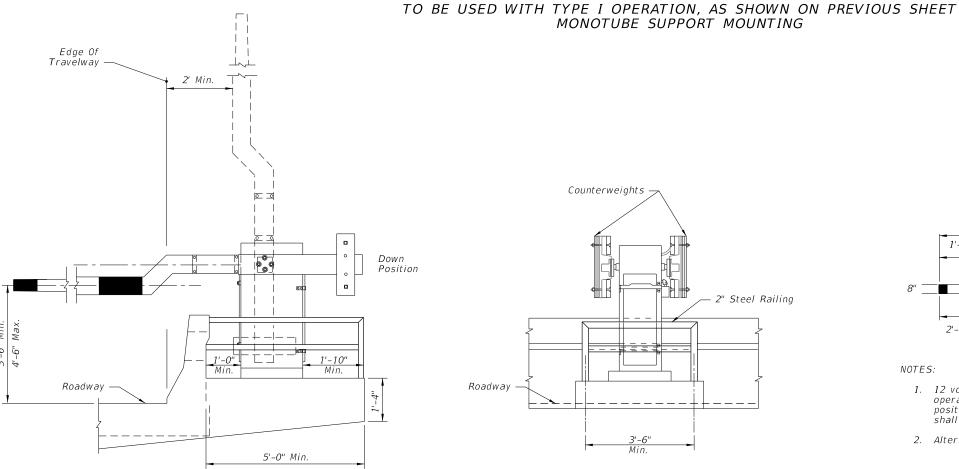
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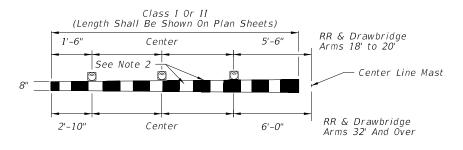
SHEET 1 of 3



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# NOTES:

- 1. 12 volt flashing red lights shall be mounted on gate arm and shall operate in the flashing mode only when gate arm is in the lower position or in the process of being lowered. The number of lights shall vary accordingly to length of the gate arm.
- 2. Alternating 16" pattern of fully reflectorized red and white stripes.

GATE & ARM DETAIL

TYPICAL LAMP PLACEMENT

**REVISION** 11/01/17

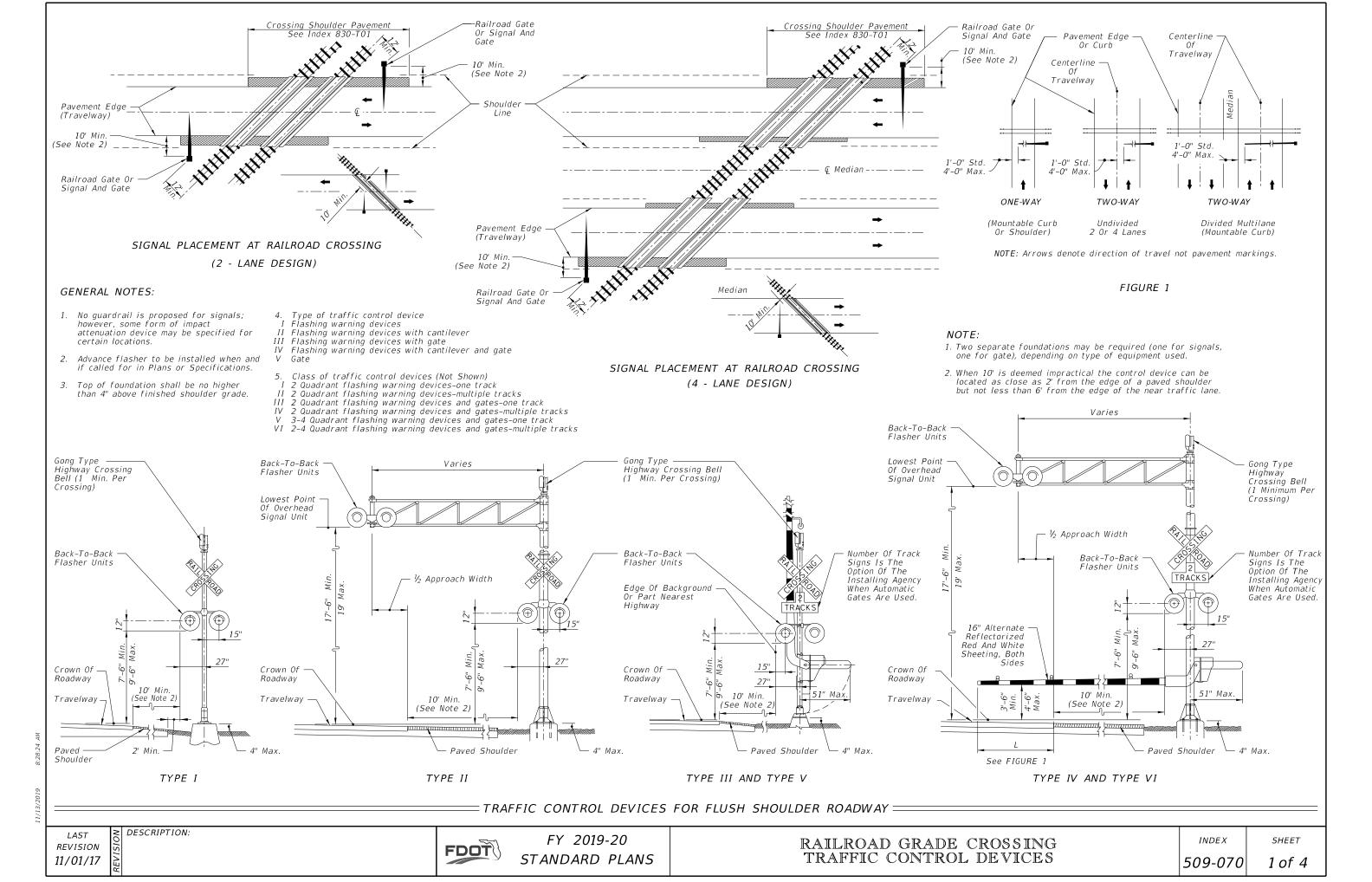
DESCRIPTION:

**FDOT** 

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TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS INDEX

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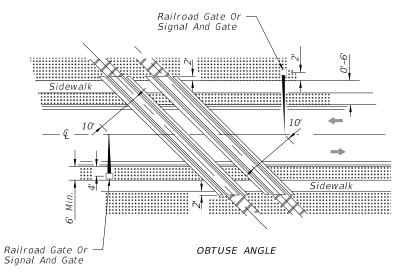
# Railroad Gate Or Signal And Gate Sidewalk Sidewalk Sidewalk Sidewalk ACUTE ANGLE (AND RIGHT ANGLE)

SIGNAL PLACEMENT AT RAILROAD CROSSING
(2 LANES, CURB & GUTTER)

DESCRIPTION:

**REVISION** 

11/01/17



SIGNAL PLACEMENT AT RAILROAD CROSSING
(2 LANES, CURB & GUTTER)

#### NOTES:

RAILROAD GRADE CROSSING

TRAFFIC CONTROL DEVICES

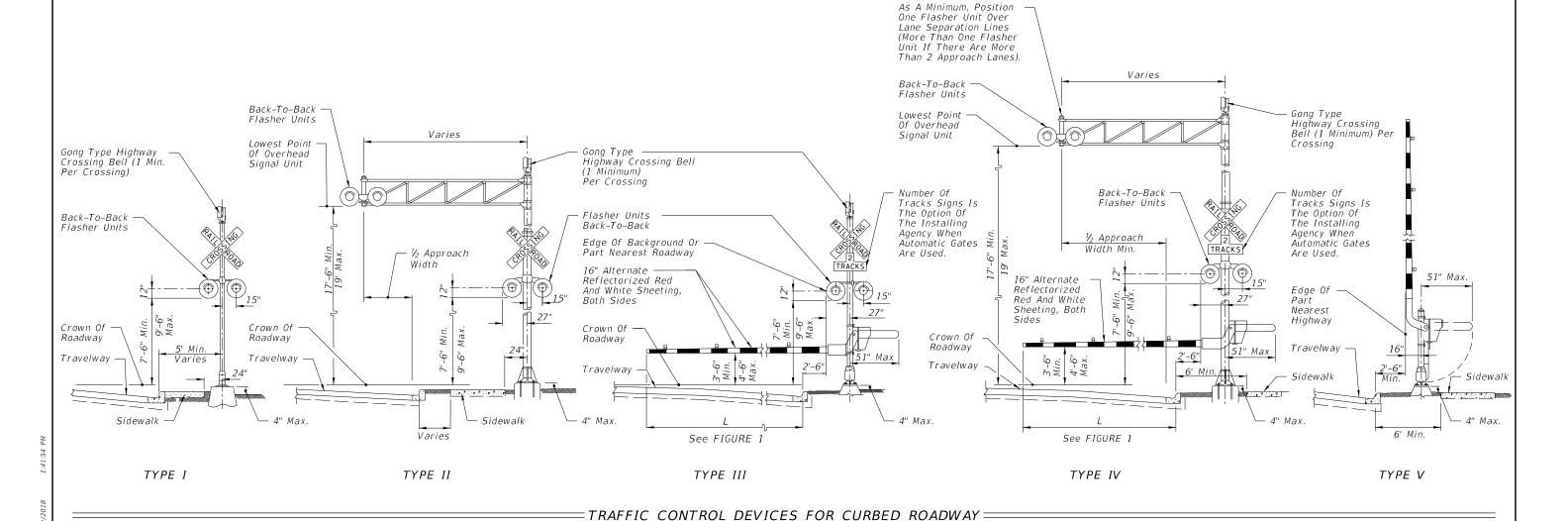
- 1. The location of flashing warning devices and stop lines shall be established based on future (or present) installation of gate with appropriate track clearances.
- 2. Where plans call for railroad traffic control devices to be installed in curbed medians, the minimum median width shall be 12'-6".
- 3. Location of railroad traffic control device is based on the distance available between face of curb & sidewalk. O' to 6' Locate device outside sidewalk. Over 6' Locate device between face of curb and sidewalk.
- 4. Stop line to be perpendicular to edge of roadway, approx. 15' from nearest rail; or 8' from and parallel to gate when present.
- 5. When a cantilevered-arm flashing warning device is used, the minimum vertical clearance shall be 17'-6" from above the Crown of Roadway to the Lowest Point of the Overhead Signal Unit.

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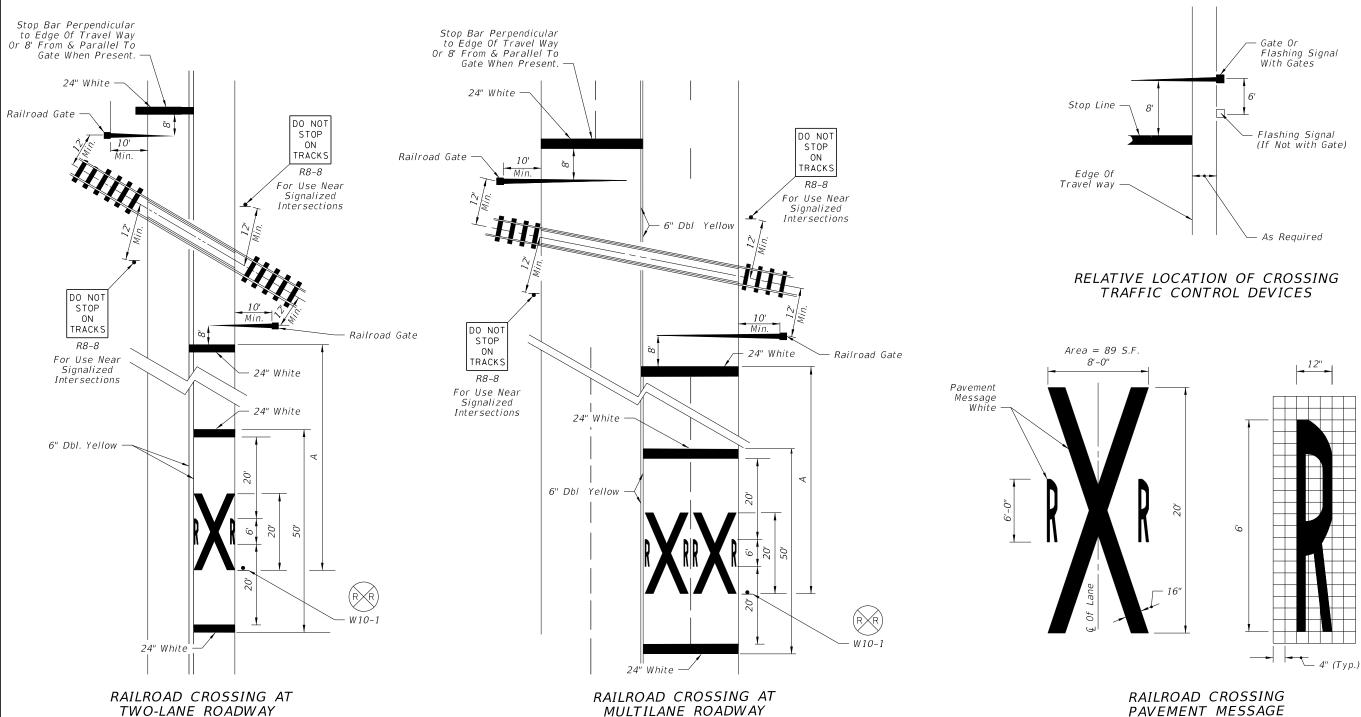
STANDARD PLANS

FDOT

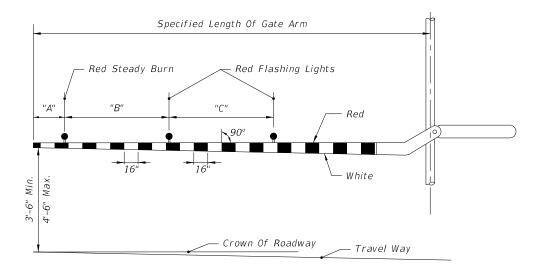
# NOTES:

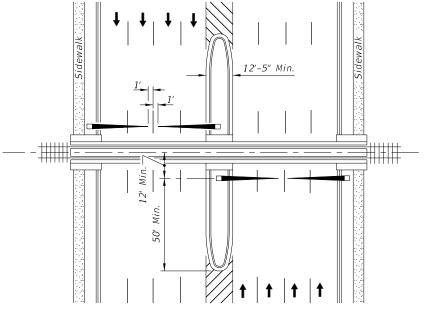
- 1. Place an additional W10-1 sign where intersections occur between the R/R pavement message and the tracks.
- 2. Place FTP-61-06 or FTP-62-06 sign 100' in advance of crossing for urban conditions and 300' in advance of crossing for rural conditions. See Index 700-102 for sign details.

Design Speed (mph)	Distance "A" (ft)
60	400
55	325
50	250
45	175
40	125
35	100
URBAN	85 Min.

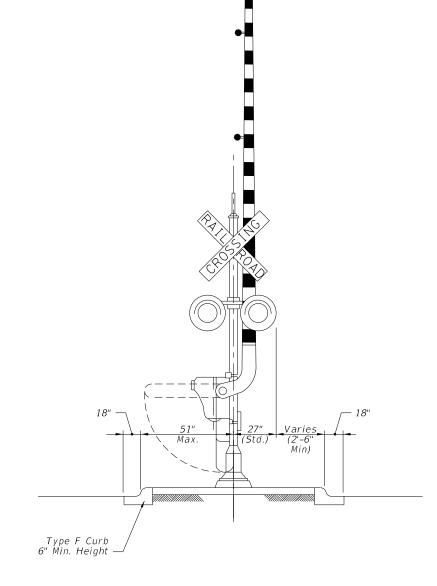


11/01/18





PLAN



MEDIAN SECTION AT SIGNAL GATES

# RAILROAD GATE ARM LIGHT SPACING

Specified Length Of Gate Arm	Dimension "A"	Dimension "B"	Dimension "C"
14 Ft.	6"	36"	5'
15 Ft.	18"	36"	5'
16-17 Ft.	24"	36"	5'
18-19 Ft.	28"	41"	5'
20-23 Ft.	28"	4'	5'
24-28 Ft.	28"	5'	5'
29-31 Ft.	36"	6'	6'
32-34 Ft.	36"	7'	7'
35-37 Ft.	36"	9'	9'
38 And Over	36"	10'	10'

NOTE: For additional information see the "Manual On Uniform Traffic Control Devices", Part 8; The "Traffic Control Handbook" , Part VIII; and AASHTO "A Policy On Geometric Design Of Streets And Highways".

MEDIAN SIGNAL GATES FOR

MULTILANE UNDIVIDED URBAN SECTIONS

(THREE OR MORE DRIVING LANES IN ONE DIRECTION, 45 MPH OR LESS)

REVISION 11/01/17

DESCRIPTION:

FDOT

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# 3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE	1 - RAILING MEMBERS	5	
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS
Post "A"	$HSS \ 2\frac{1}{2} \times 1\frac{1}{2} \times \frac{1}{8}$	2.50" x 1.50"	0.125"
Post "B"	HSS 2½ x 1½ x¾ ₁₆	2.50" x 1.50"	0.188"
Tan Dail	2½" NPS (Sch. 10)	2.875"	0.120"
Top Rail	HSS 3.000 x 0.120	3.000"	0.120"
F. d. 11	2½" NPS (Sch. 10)	2.875"	0.120"
End Hoops	HSS 3.000 x 0.120	3.000"	0.120"
Top Rail Joint/Splice Sleeves	HSS 2.500 x 0.125	2.500"	0.125"
Intermediate & Bottom Rail	HSS 2 x 2 x ³ / ₁₆	2.00" x 2.00"	0.188" (1)
Int. & Bottom Rail Post Connection Sleeve	HSS 1.500 x 0.125	1.500"	0.125" (1)
Handrail Joint/Enlice Classes	1" NPS (Sch. 40)	1.315"	0.133"
Handrail Joint/Splice Sleeves	HSS 1.500 x 0.125	1.500"	0.125"
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"
Handrail Support Bar	¾" Ø Round Bar	0.750"	N/A
Pickets (Type 1 Infill Panel)	³¼" Ø Round Bar	0.750"	N/A
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies

#### TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be  $1\frac{1}{4}$ " NPS (Sch. 40).

NOTES =

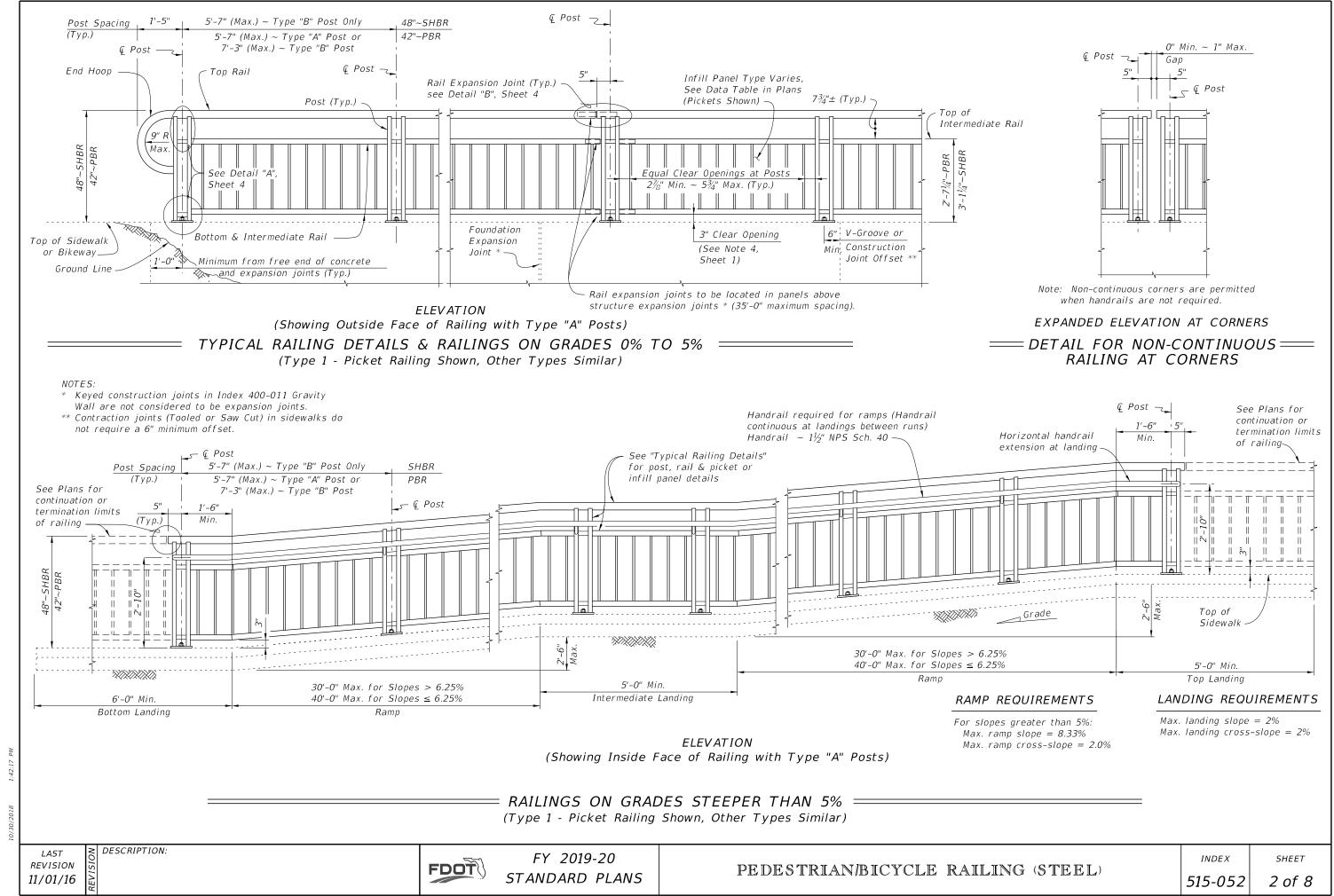
- 1. Shop Drawings are required; see Specification Section 515
- 2. For bridge mounted railings work this Index with Index 515-051 Bridge Bicycle/Pedestrian Railing
- - A. Pipe Rails and Pickets: ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) and ASTM A36 for bars.
  - B. Structural Tube: ASTM A500 Grade A, B, C, or D or ASTM A501
  - C. Steel Plate: ASTM A36 or ASTM A709 Grade 36
  - D. U-Channels and filler plates: ASTM A36 or ASTM A1011 (Grade 36).
  - E. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
  - F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962.
    - a. Hex Head Bolts: ASTM A 307
      - 1.  $\frac{1}{8}$ " diameter single bolt option, Grade 36
      - 2.  $\frac{1}{16}$ " four bolt option, Grade 55
    - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
    - c. Hex Nuts: ASTM A563
    - d. Flat Washers: ASTM F436
    - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
  - G. Shims: ASTM B209 Alloy 6061
  - H. Bearing Pads: 1/8" Plain, Fabric Reinforced or Fabric Laminated pads that meet the requirements of Specification Section 932 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5% for standard installations and 3% when a 4" sphere requirement is indicated in the Data Tables.
- 5. Maximum spacing between expansion joints is 40'-0". Locate an Expansion Joint between the posts on either side of the Deck
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K".
- 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for:
  - A. Grades Steeper than 5%,
  - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

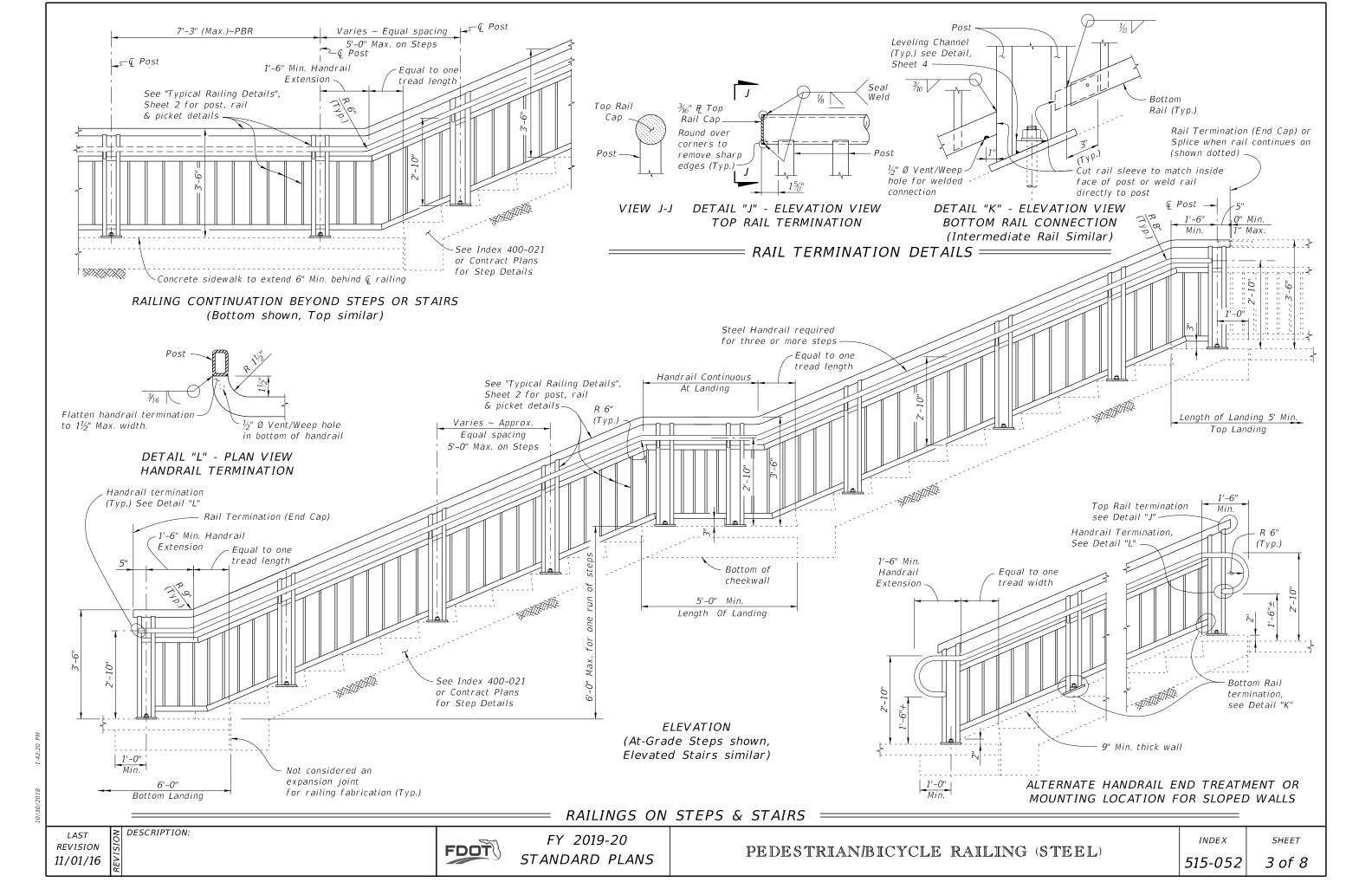
**REVISION** 11/01/18

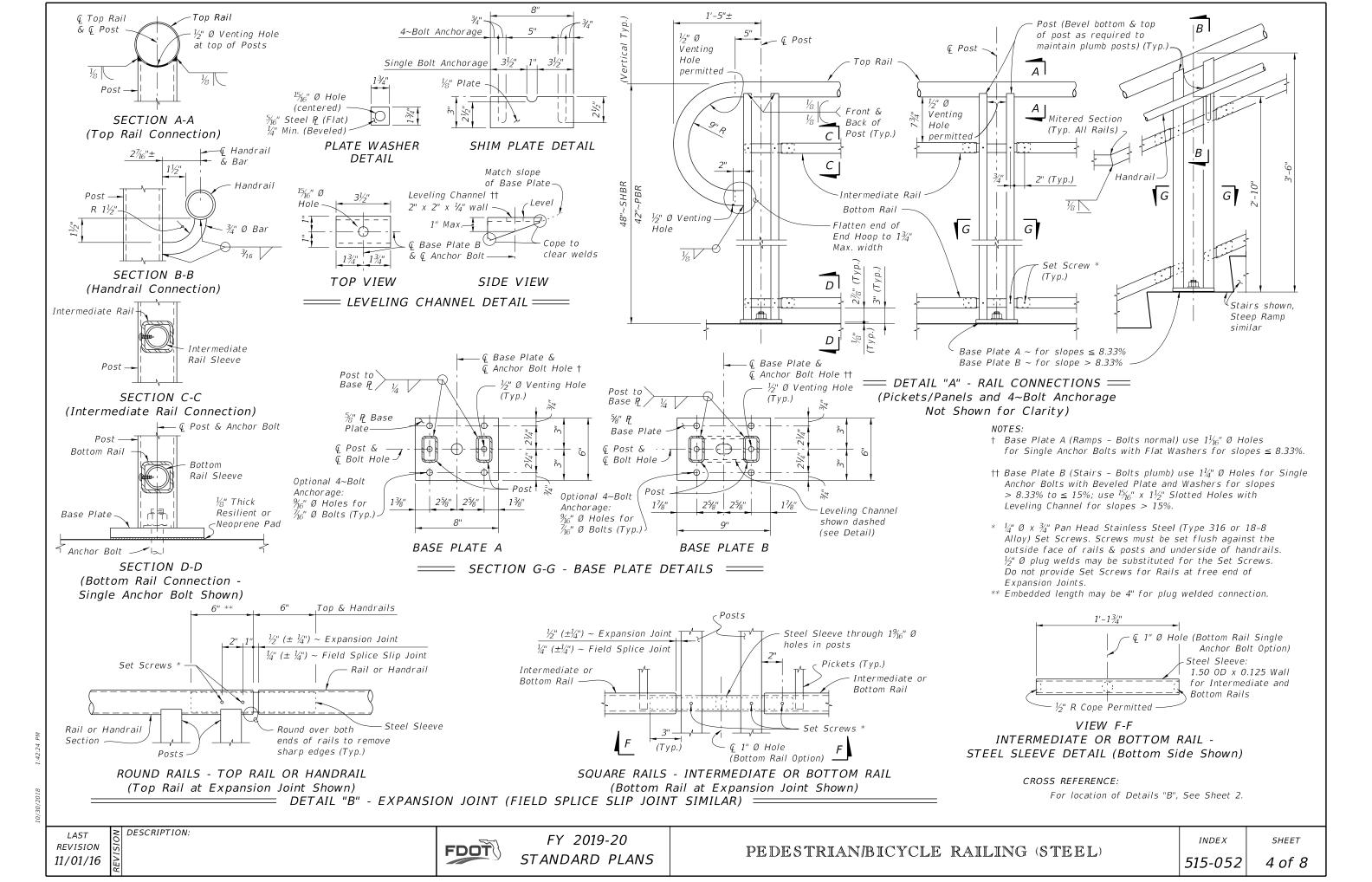
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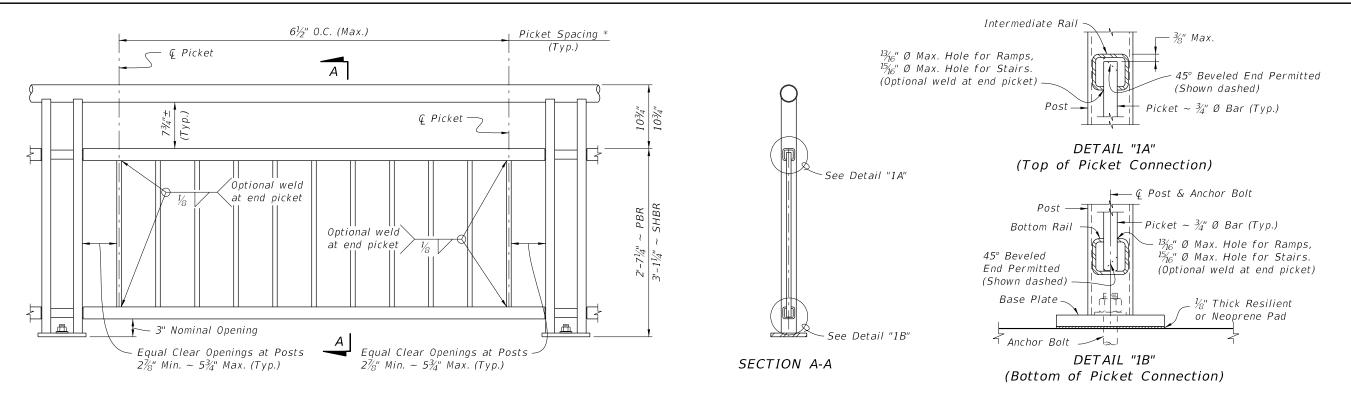


FY 2019-20 STANDARD PLANS PEDESTRIAN/BICYCLE RAILING (STEEL)





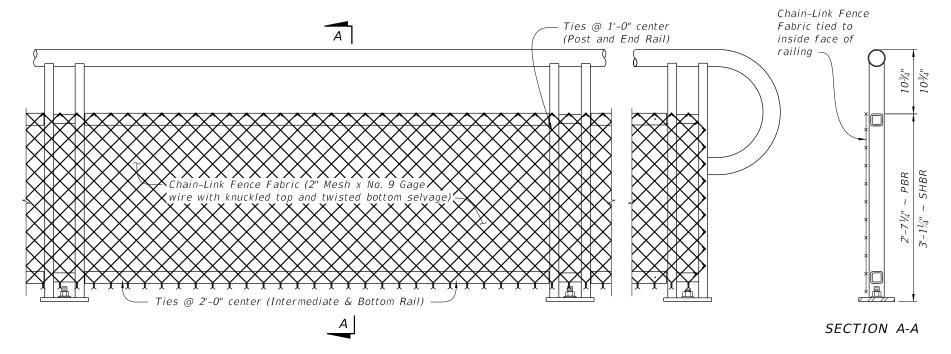




# TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of  $6\frac{1}{2}$ " centers is based on a  $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a  $4\frac{1}{2}$ " picket spacing may be required. See Note 4 (Sheet 1).



Chain-Link Fence Fabric (2" mesh with	A 392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating
twisted bottom and knuckled top selvage)	A 491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)
, , ,	F 668	Polyvinyl Chloride (PVC) Coated Steel - No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.
Tie Wires	F 626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.
Tension Bars	F 626	$rac{3}{16}$ " (Min. thickness) x $rac{3}{4}$ " (Min. width) x 2'–3' (Min. height) Steel Bars
Miscellaneous Fence Components	F 626	Zinc-Coated Steel

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS

COMPONENT INFORMATION

**ASTM** 

#### CHAIN-LINK PANEL NOTE:

COMPONENT

Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES:

1. See Plans for Infill Panel option required.

DESCRIPTION: **REVISION** 11/01/16

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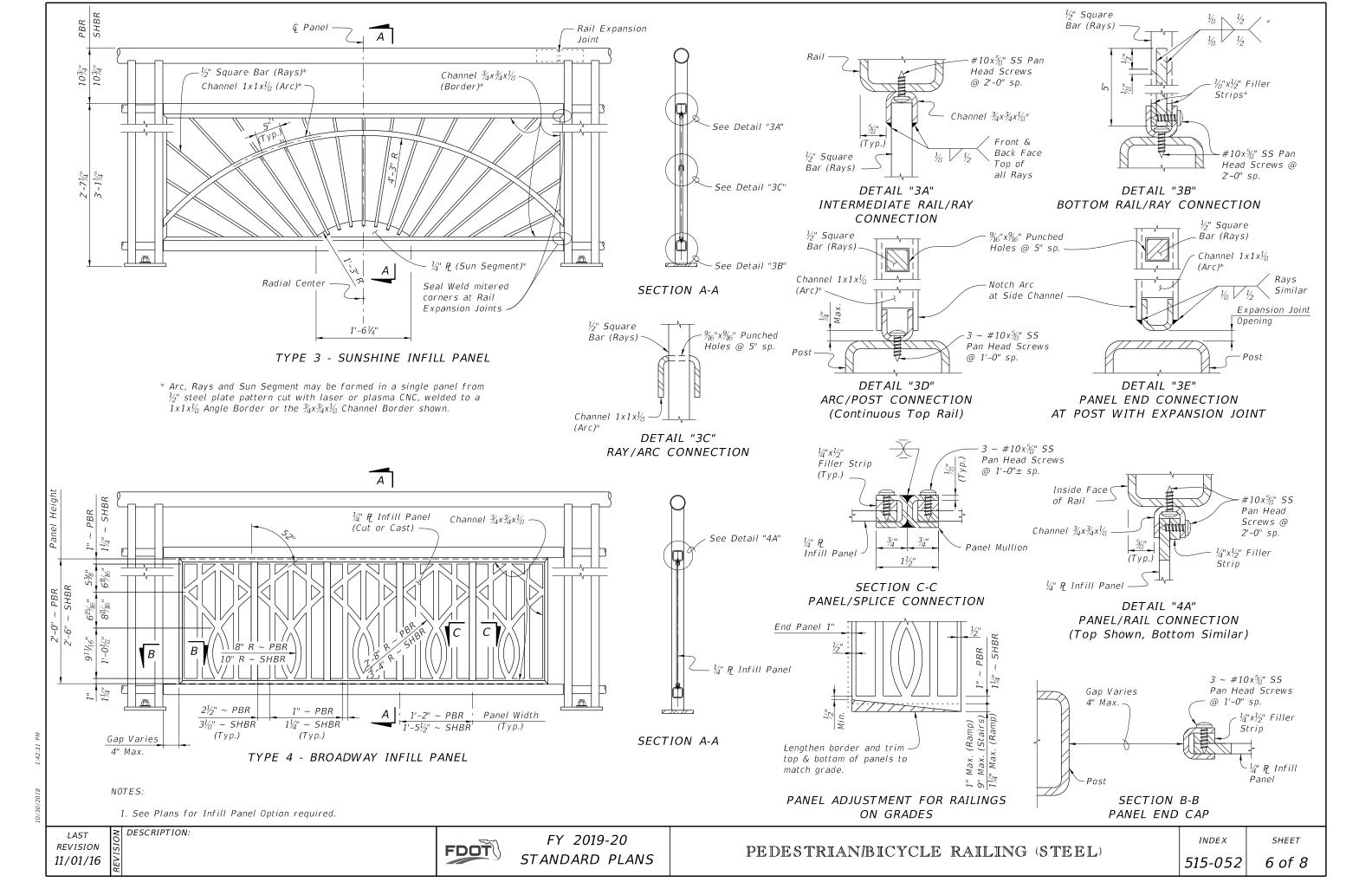
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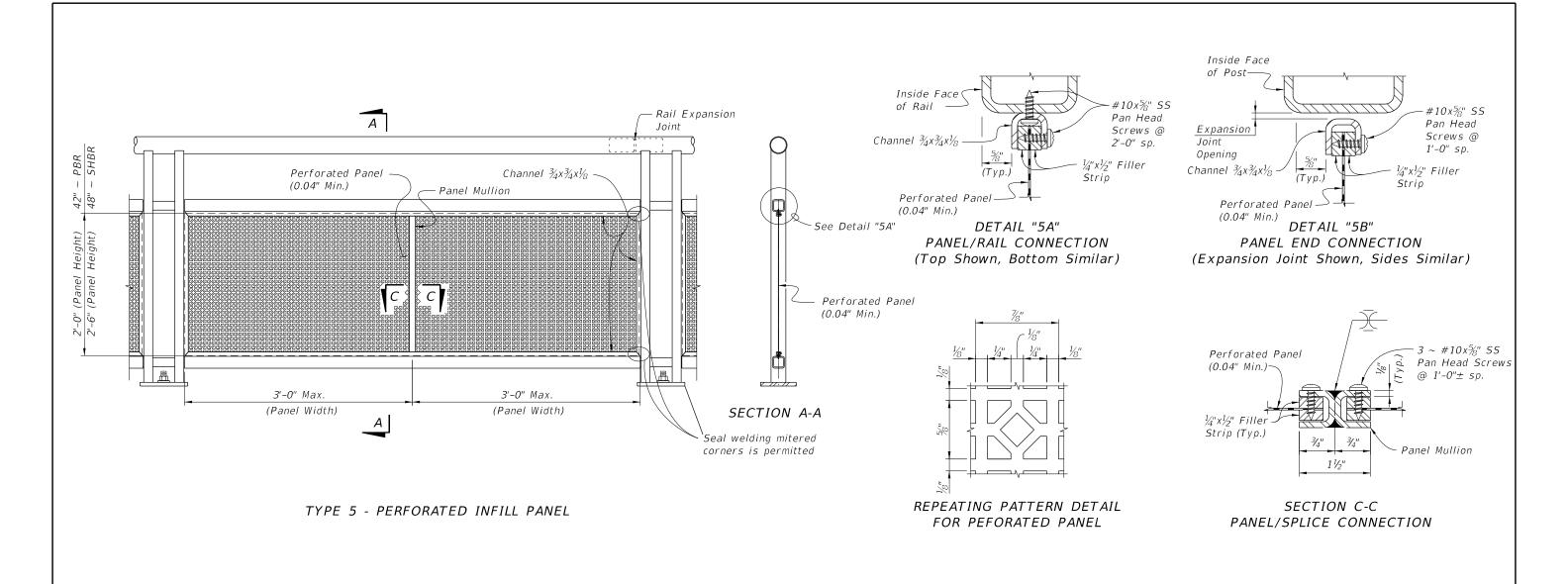
PEDESTRIAN/BICYCLE RAILING (STEEL)

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No.



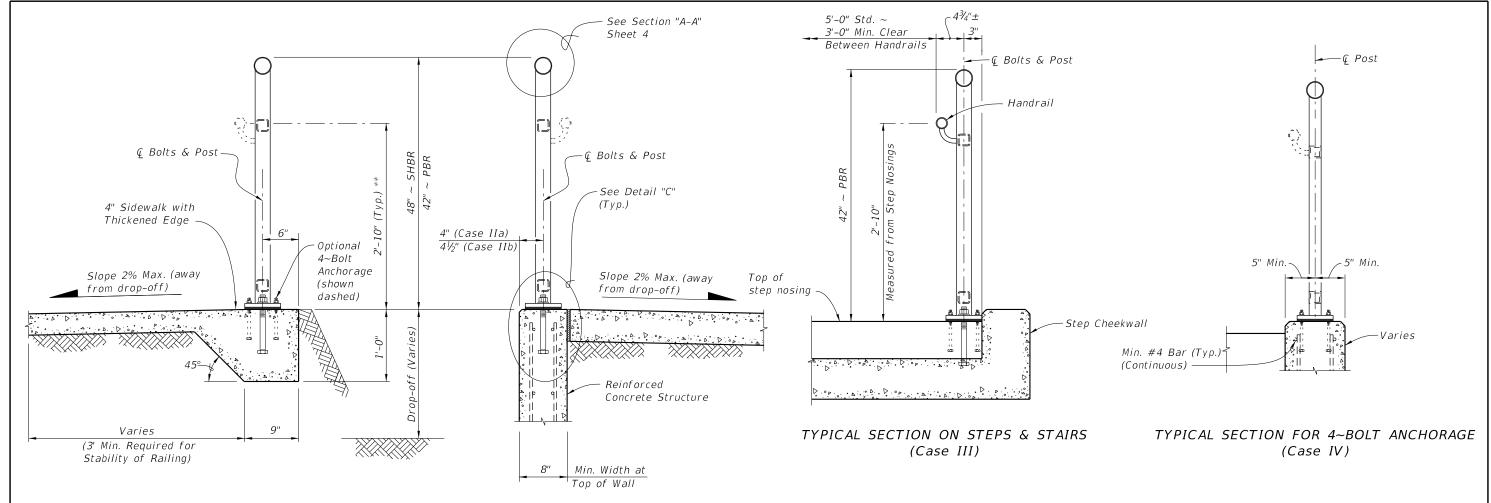


NOTES:

1. See Plans for Infill Panel Type required.

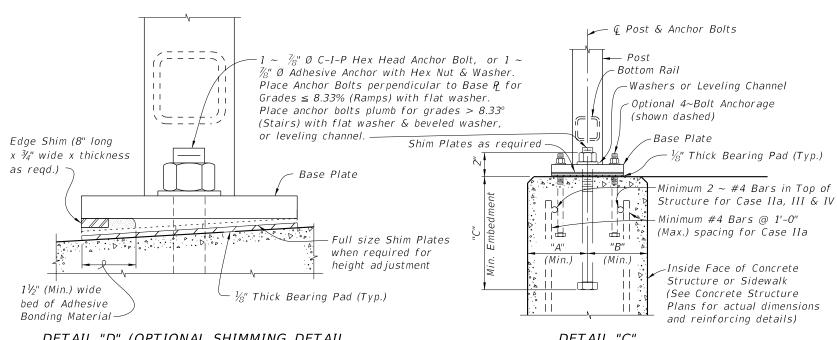
DESCRIPTION: REVISION 11/01/16

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#### TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

#### TYPICAL SECTION ON RETAINING WALL (Case II)



ANCHOR BOLT TABLE							
CASE	STRUCTURE TYPE	DIMENSIONS			ANCHOR LENGTH		ANGUOR
CASE		A Edge Dist.	B Edge Dist.	C Embedment	C-I-P Hex Head Bolt		ANCHOR SIZE
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	%" Ø
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø
IIb	Gravity Wall Index 400-011	41/2"	3½" @ top	1'-0" *	1'-1½"	1'-2"	%" Ø
III	Step Cheekwall	41/2"	4½"	9"	101/2"	11"	%" Ø
IV	Varies	5"	5"	5"	6½"	7"	7∕16" Ø

- * Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".
- ** When required; measured from top of sidewalk.

DETAIL "D" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

DESCRIPTION:

DETAIL "C" (Cast-In-Place Anchor Bolts shown, Adhesive Anchors similar)

**REVISION** 11/01/16

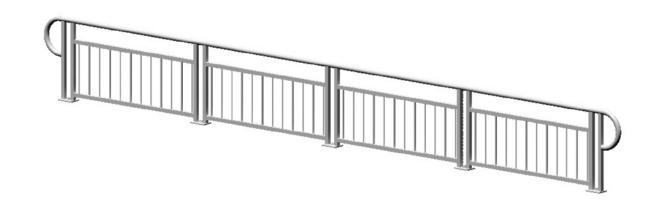
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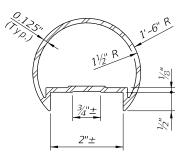


#### 3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS				
MEMBER	ALLOY ⁽¹⁾	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS
Posts (Type "A" & "B")	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250"
Posts (Type "C")	6061-T6	Extrusion 1½x2½x0.125	1.50" x 2.50"	0.125"
Top Plate (Type "C")	6061-T6	Extrusion (See Details)	2¾" x 7"	Varies
Torr. Dall	6061 T6	2½" NPS (Sch. 10)	2.875"	0.120"
Top Rail	6061-T6	3" Round Top Cap Rail	3.000"	0.125"
5.1.4	6063-T5	2½" NPS (Sch. 10)	2.875"	0.120"
End Hoops		3.00 OD x 0.125 Wall	3.000"	0.125"
T. D. I. L. I. G. I. G.	6063-T5	2.50 OD x 0.125 Wall	2.500"	0.125"
Top Rail Joint/Splice Sleeves		Top Cap Rail Inner Sleeve	2.800"	0.090"
Intermediate & Bottom Rail	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250" (2)
Int. & Bottom Rail Post Connection Sleeve	6063-T5	1.50 OD x 0.125 Wall ⁽³⁾	1.500"	0.125"
	6063-T5	1" NPS (Sch. 40)	1.315"	0.133"
Handrail Joint/Splice Sleeves	6063-T5	1.50 OD x 0.125 Wall	1.500"	0.125"
Handrails	6061-T6	1½" NPS (Sch. 40)	1.900"	0.145"
Handrail Support Bar	6061-T6	¾" Ø Round Bar	0.750"	N/A
Pickets (Type 1 Infill Panel)	6061-T6	¾" Ø Round Bar	0.750"	N/A
Infill Panel Members (Types 2 - 5)	6063-T5	Varies (See Details)	Varies	Varies

#### TABLE 1 NOTES:

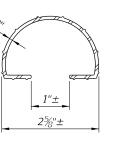
- (1) Alloy 6061-T6 or 6063-T52 & T6 may be substituted for Alloy 6063-T5.
- (2) 0.188" wall thickness permitted for rails with post spacings less than 5'-9".
- (3) 1" NPS (Sch. 40) non-slit rail sleeves may be substituted when welded connection Detail "K" is utilized.



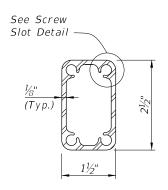
1"±

3" ROUND TOP CAP RAIL TOP CAP RAIL INNER SPLICE SLEEVE

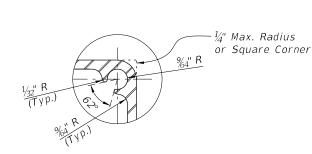
ALTERNATE TOP RAIL SECTION =



ALTERNATIVE BOTTOM & INTERMEDIATE RAIL SECTION FOR TYPE 3, 4 & 5 RAILINGS



POST TYPE "C" SCREW SLOT SECTION



SCREW SLOT DETAIL

#### NOTES:

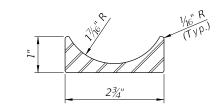
- 1. Shop Drawings are required, see Specification Section 515.
- 2. For bridge mounted railings, work this Index with Index 515-061 Bridge Bicycle/Pedestrian Railing (Aluminum)
- 3. Materials:
  - A. Structural Extrusions, Tube, Pipe and Bars: Table 1 and ASTM B221 or ASTM B429
    - a. Top, bottom and intermediate rail corner bends with maximum 4'-0" post spacing may be Alloy 6063-T6
  - B. Base Plates and Rail Caps: ASTM B209 Alloy 6061-T6
  - C. Perforated panels (Type 5) Alloy 3003-H14
  - D. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
  - E. Aluminum screws: Alloy 2024-T4 or 7075-T73
  - F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962.
    - a. Hex Head Bolts: ASTM A 307
      - 1. %" diameter single bolt option, Grade 36
      - 2.  $\frac{7}{16}$ " diameter four bolt option, Grade 55
    - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
    - c. Hex Nuts: ASTM A563
    - d. Flat Washers: ASTM F436
    - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
  - G. Shims: ASTM B209 Alloy 6061 or 6063
  - H. Bearing Pads: Provide  $\frac{1}{2}$ " thick Plain, Fabric Reinforced or Fabric Laminated Bearing Pads meeting the requirements of Specification Section 932 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5%" for standard installations and 3%" when a 4" sphere requirement is indicated in the Data Tables.
- 5. Locate railing expansion Joints between the posts on either side of
- the deck expansion joint. Maximum spacing between expansion joints is 35'-0".
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K" for Post Type "A" & "B".
- 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for:
  - A. Grades Steeper than 5%,
  - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

#### CROSS REFERENCES:

Detail "A", Sheet 4

Detail "B", Sheet 4

Detail "K", Sheet 3



OPTIONAL TOP PLATE EXTRUSION SECTION (POST TYPE "C")

DESCRIPTION: LAST

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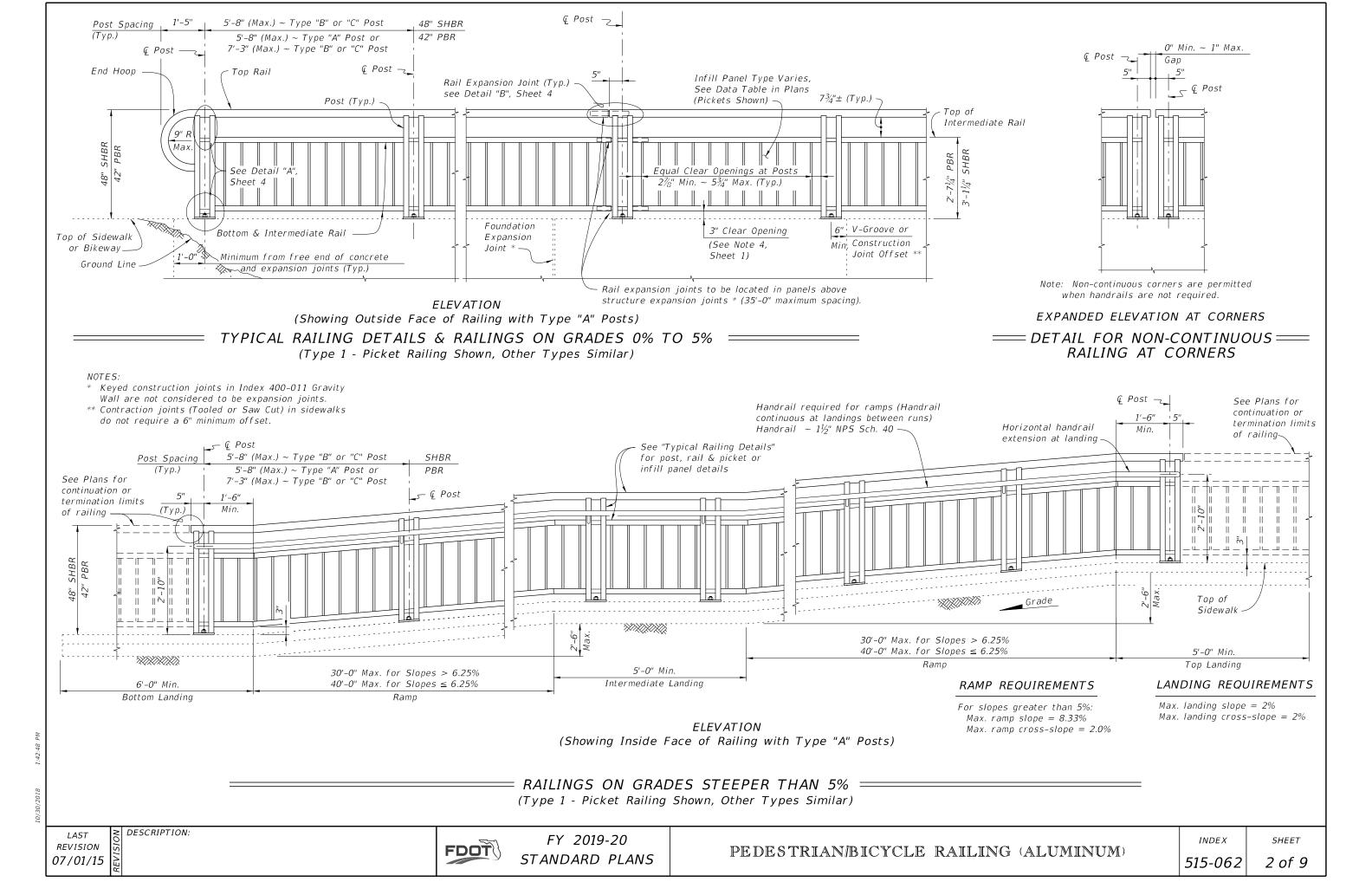
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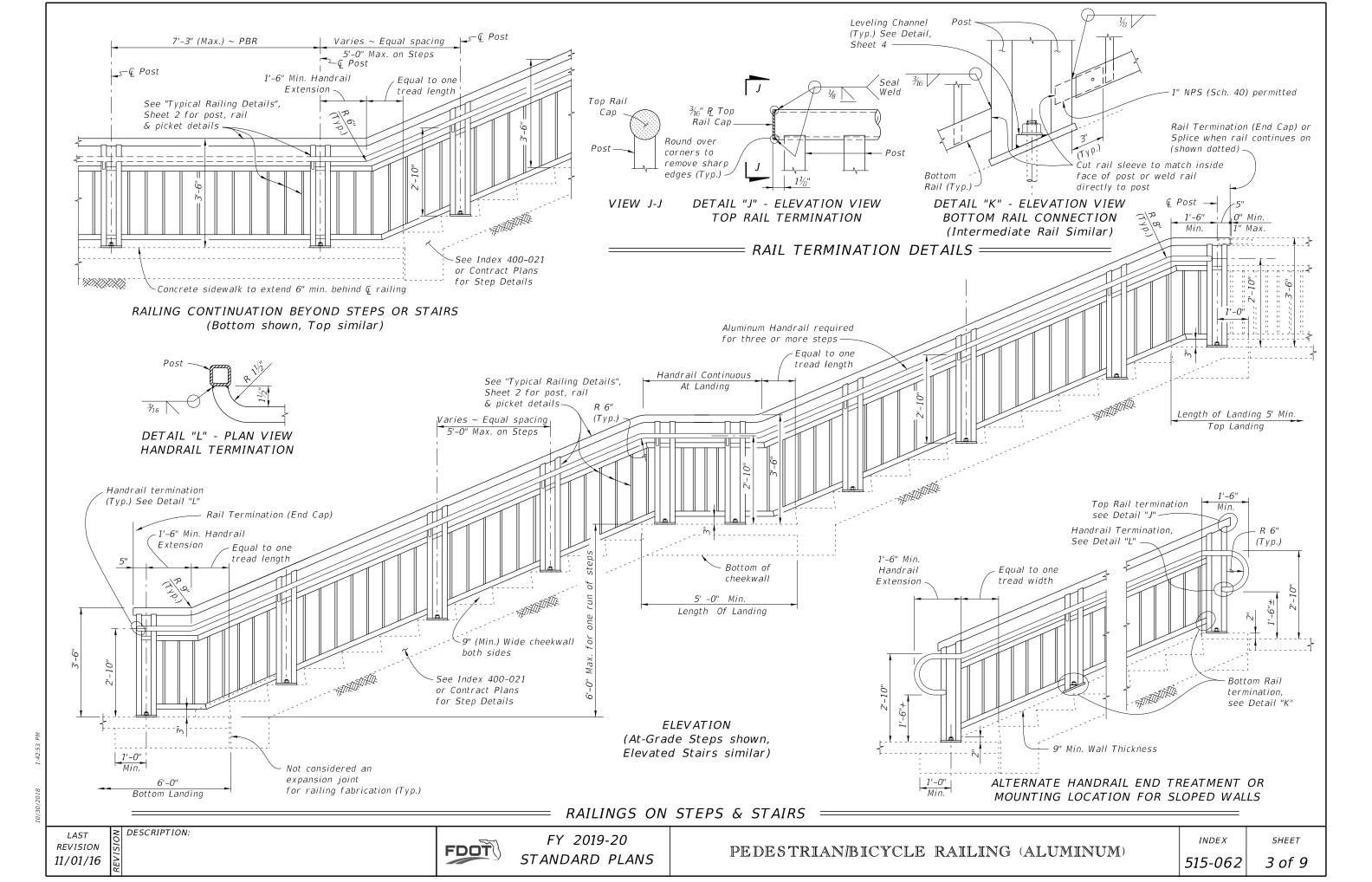
SHEET

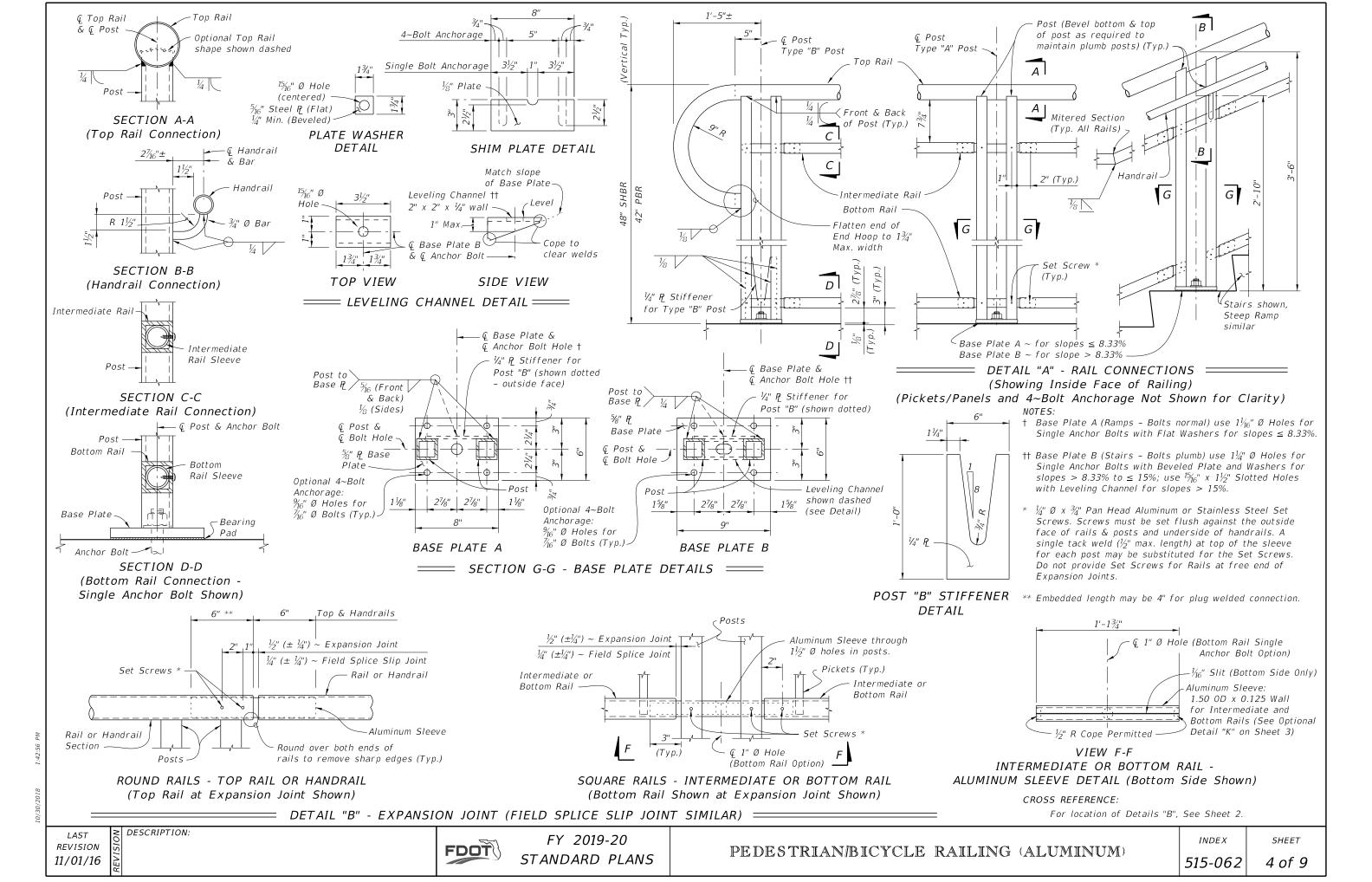
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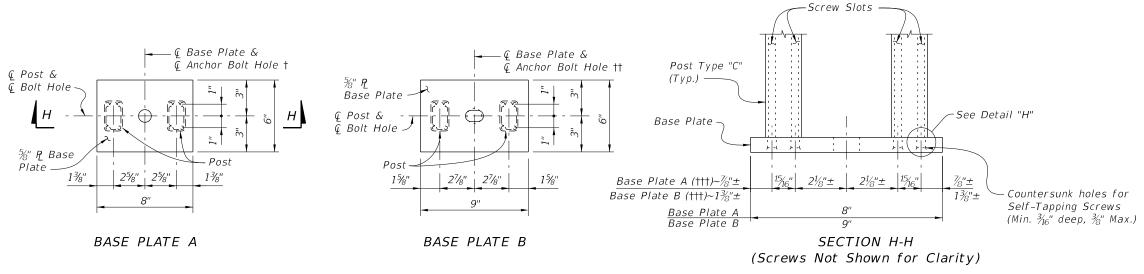
**REVISION** 11/01/18

PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

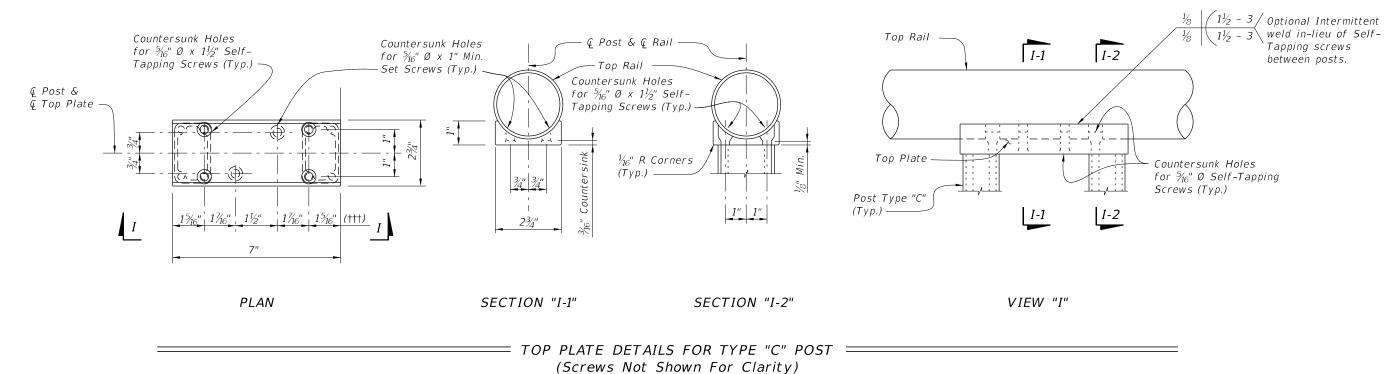












- See Sheet 4 for Notes.
- See Sheet 4 for Notes.
- Length varies for beveled posts on grades. Holes must be drilled plumb to align with screw slot.

DESCRIPTION: REVISION 11/01/16

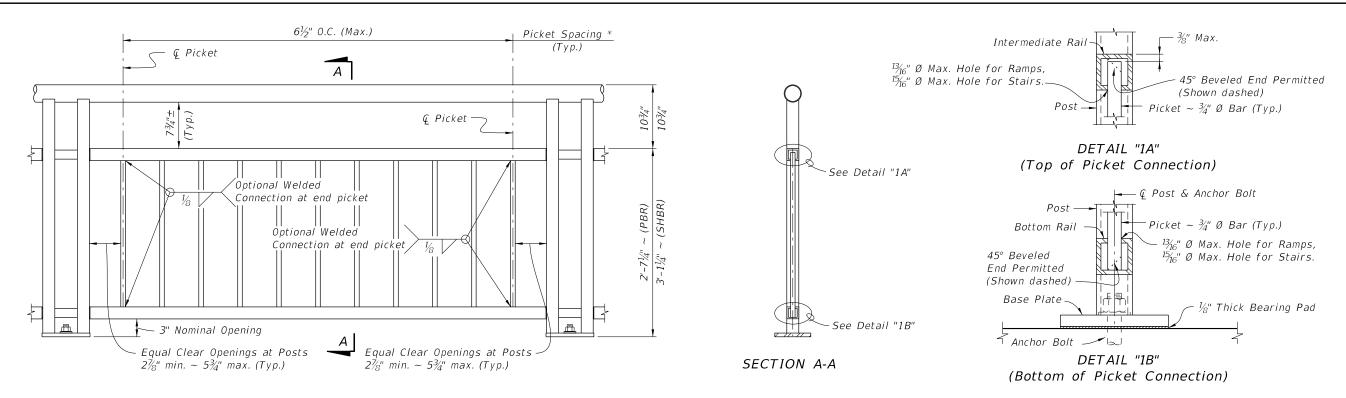
FDOT

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PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

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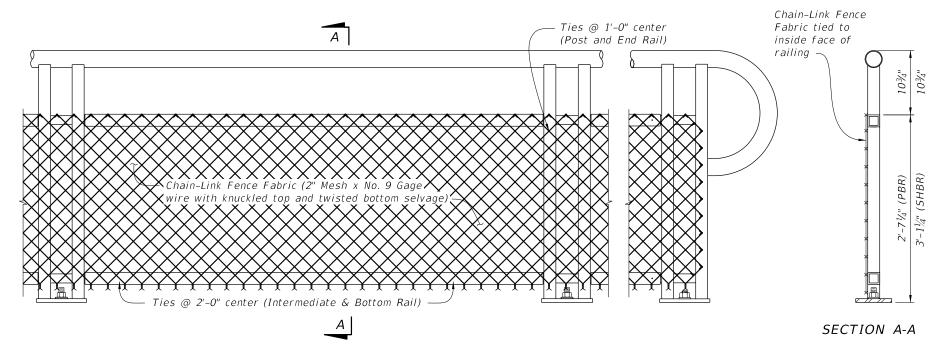
SHEET



#### TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of  $6\frac{1}{2}$ " centers is based on a  $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a  $4\frac{1}{2}$ " picket spacing may be required. See Note 4 (Sheet 1).



CHAIN-LINK	PANEL NOTE:	

COMPONENT

Chain-Link Fence

Fabric (2" mesh with

knuckled top selvage)

twisted bottom and

Tie Wires

Tension Bars

Components

Miscellaneous Fence

Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS

**ASTM** 

A392

A491

F668

F626

F626

F626

TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES:

1. See Plans for Infill Panel option required.

DESCRIPTION: **REVISION** 11/01/16

**FDOT** 

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PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

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SHEET

COMPONENT INFORMATION

Polyvinyl Chloride (PVC) Coated Steel - No.

9 gage Zinc-Coated Wire (metallic-coated

Zinc-Coated Steel Wire - No. 9 gage with

coating to match Chain-Link Fence Fabric.  $\frac{3}{16}$ " (min. thickness) x  $\frac{3}{4}$ " (min. width)

Zinc-Coated Steel - No. 9 gage (coated

Aluminum-Coated Steel - No. 9 gage

core wire diameter) ~ See Plans for

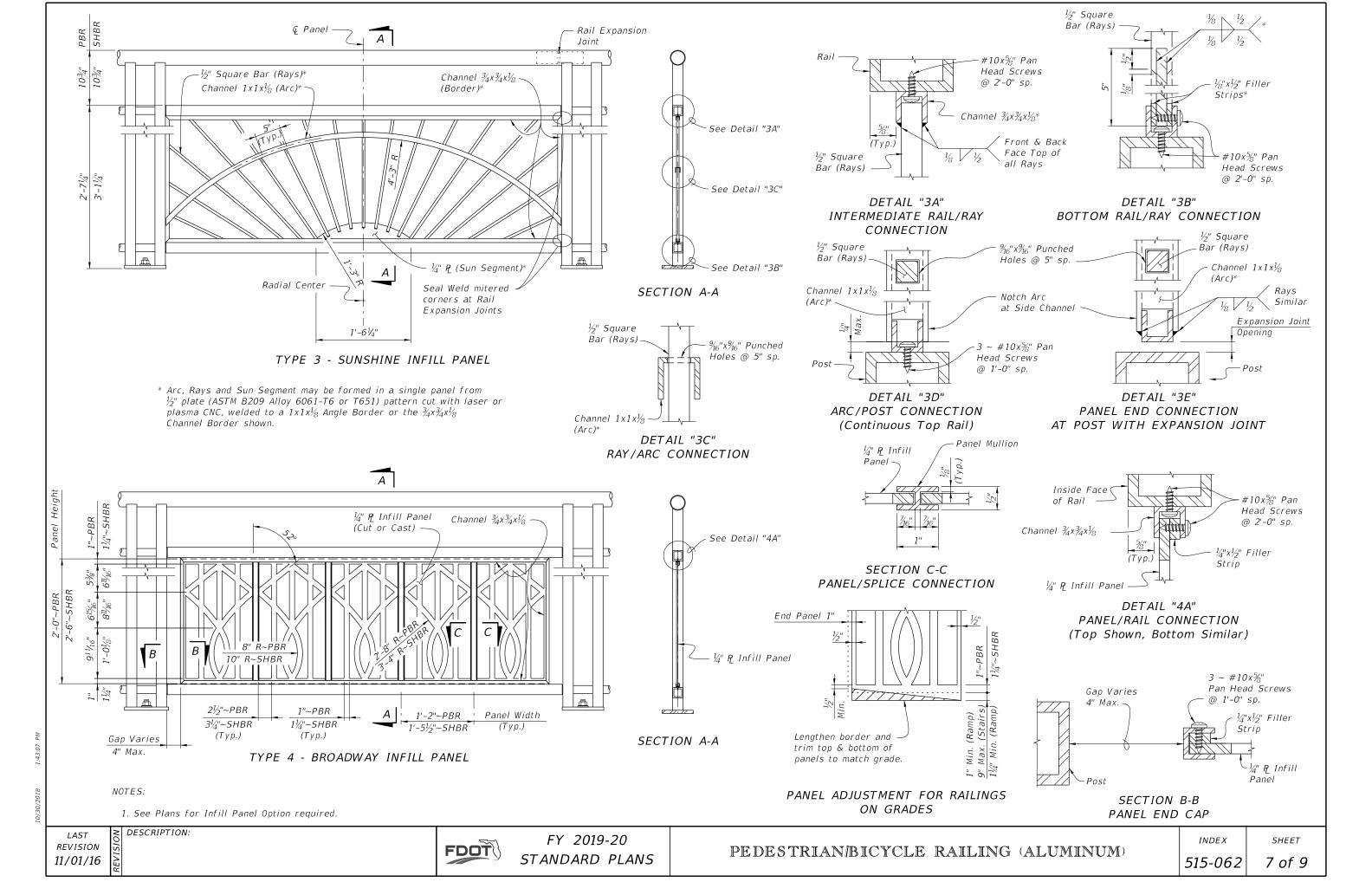
wire diameter), Class 2 Coating

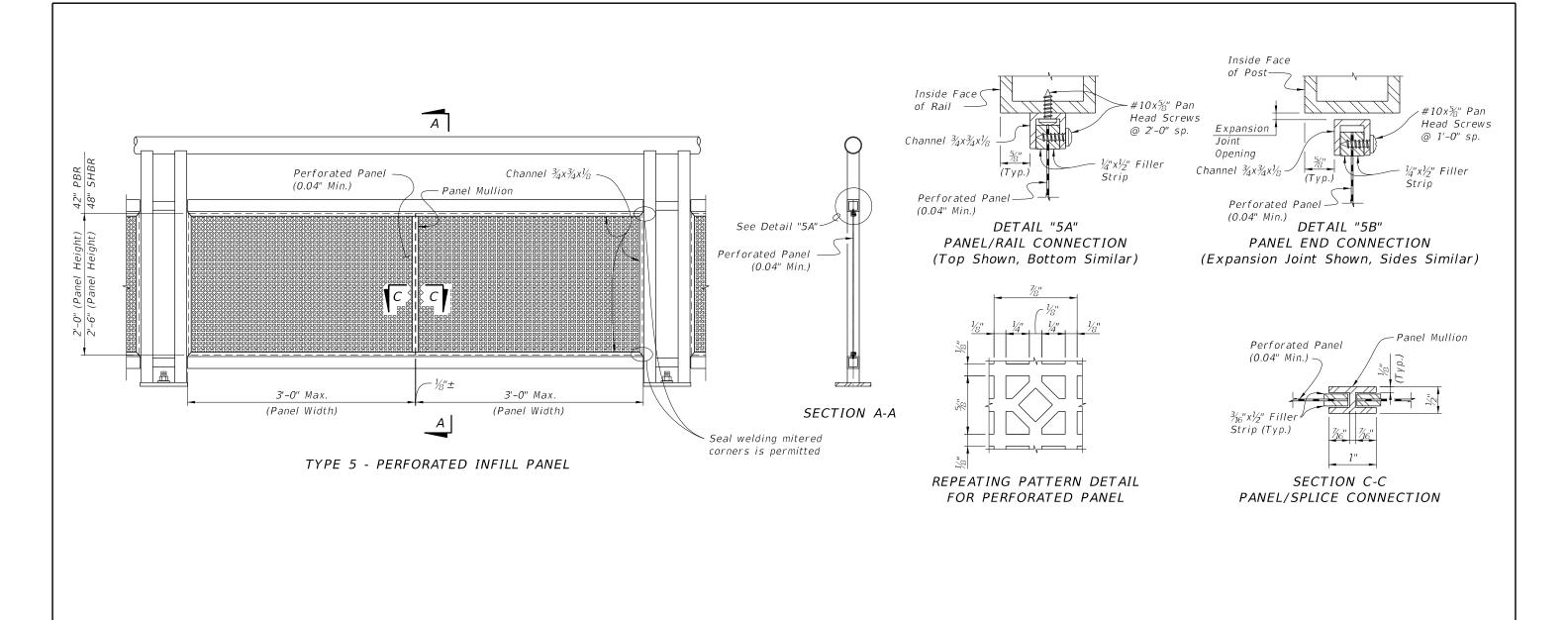
x 2'-3' (min. height) Steel Bars

(coated wire diameter)

specified color of PVC.

Zinc-Coated Steel



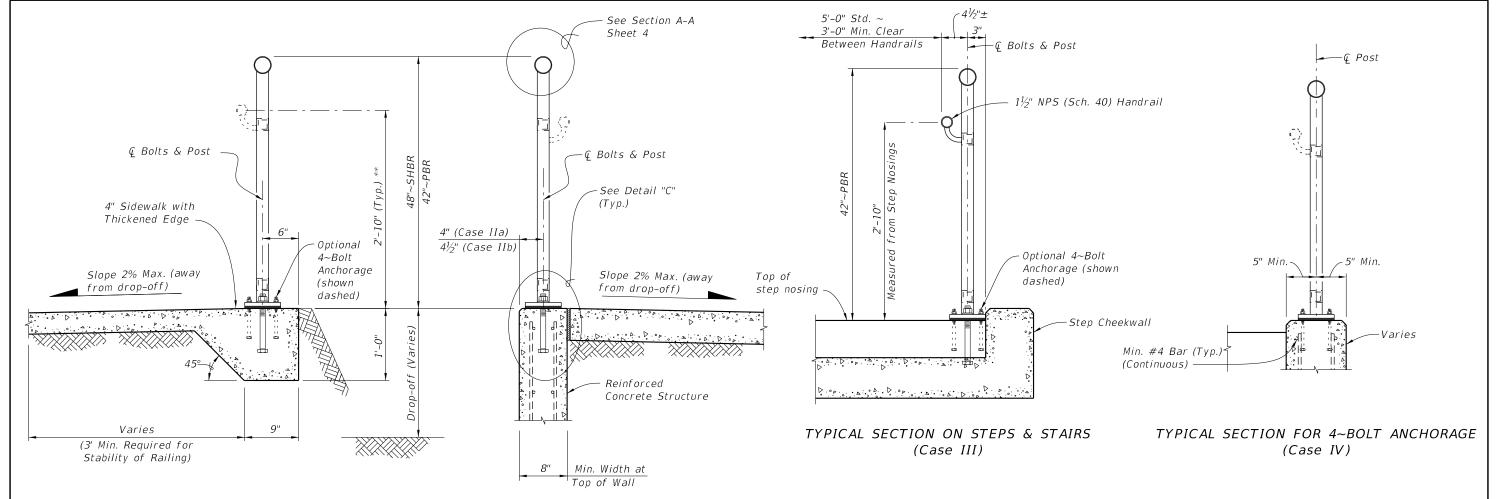


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LAST REVISION 11/01/16

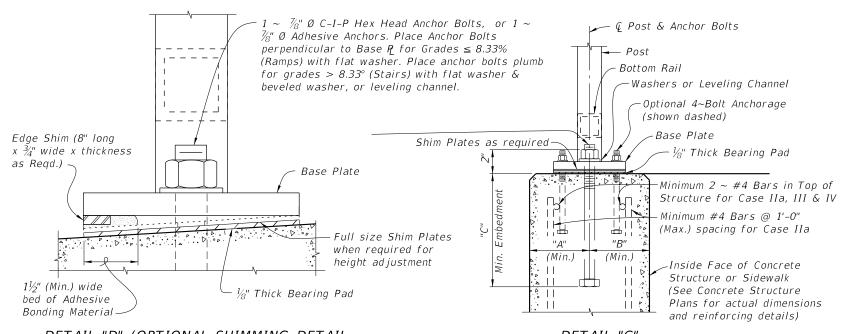
DESCRIPTION:

FDOT



TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

TYPICAL SECTION ON RETAINING WALL (Case II)



	ANCHOR BOLT TABLE						
CASE	STRUCTURE TYPE	DIMENSIONS			ANCHOR LENGTH		ANCHOR
CASE		"A" Edge Dist.	"B" Edge Dist.	"C" Embedment	C.I.P Hex Head Bolt	Adhesive Anchor	SIZE
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	½" Ø
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø
IIb	Gravity Wall Index 400-011	4½"	3½" @ top	1'-0" *	1'-1½"	1'-2"	%" Ø
III	Step Cheekwall	4½"	4½"	9"	10½"	11"	%" Ø
IV	Varies	5"	5"	5"	6½"	7"	7₁6" Ø

^{*} Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".

DETAIL "D" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

DESCRIPTION:

DETAIL "C" (Cast-In-Place Anchor Bolts shown, Adhesive Anchors similar)

**REVISION** 11/01/16

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^{**} When required; measured from top of sidewalk (Typ.)

#### NOTES:

- 1. Shop Drawings are required.
- 2. Work with Specification 515.
- 3. <u>Materials:</u>
- A. Pan Head Set Screws: Aluminum Alloy 2024-74 or 7075-T73 or Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM B209, Alloy 6061-T6
- C. Structural Pipe Tube and Bars: ASTM B221 or ASTM B429, Alloy 6061-T6
- D. End Rails 90° bends and corner bends with a maximum 4 foot spacing: Alloy 6063-T5 is permitted.

RAILING MEMBER DIMENSIONS TABLE				
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS	
Posts	2" NPS (Sch. 40)	2.375"	0.154"	
Rails	2" NPS (Sch. 40)	2.375"	0.154"	
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) 1.50 ODx0.125 Wall	1.315" 1.500"	0.133" 0.125"	
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A	

- E. Galvanized Steel Fasteners:
- a. Hex Head Bolts: ASTM A 307 Type 1 or ASTM F1554 Grade 36
- b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods
- c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- F. Aluminum Shims: ASTM B209, Alloy 6061
- G. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specifications 515 & 932.
- 4. Fabrication:
  - A. Place expansion joints at a maximum of 30'-0"spacing
- B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts. C. Continuity field splice (Detail "E"); only use to make the railing continuous for unforeseen field adjustments

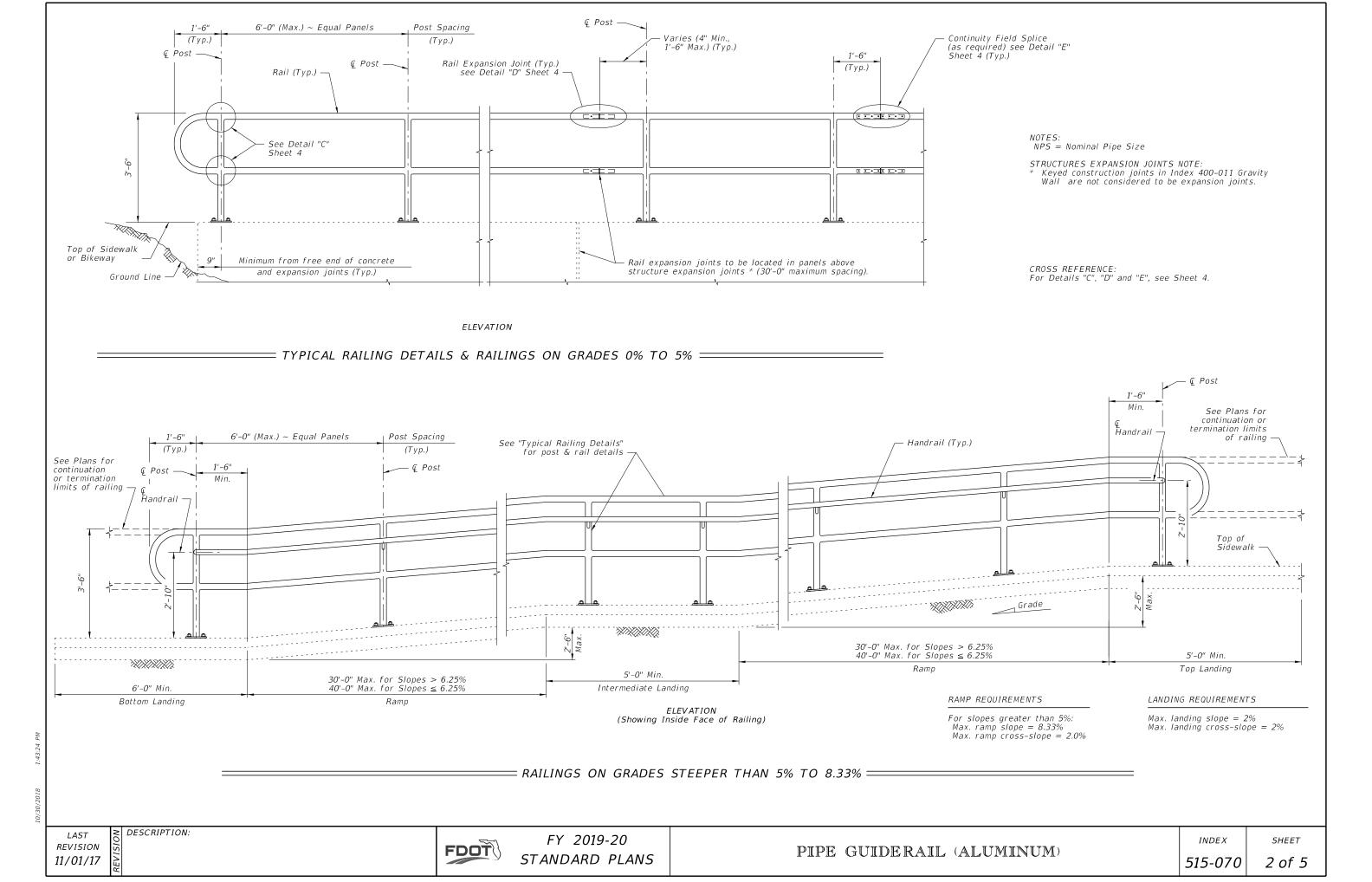
- D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9" bend radius or terminated at adjoining sections with a standard end hoop when handrails are not required.

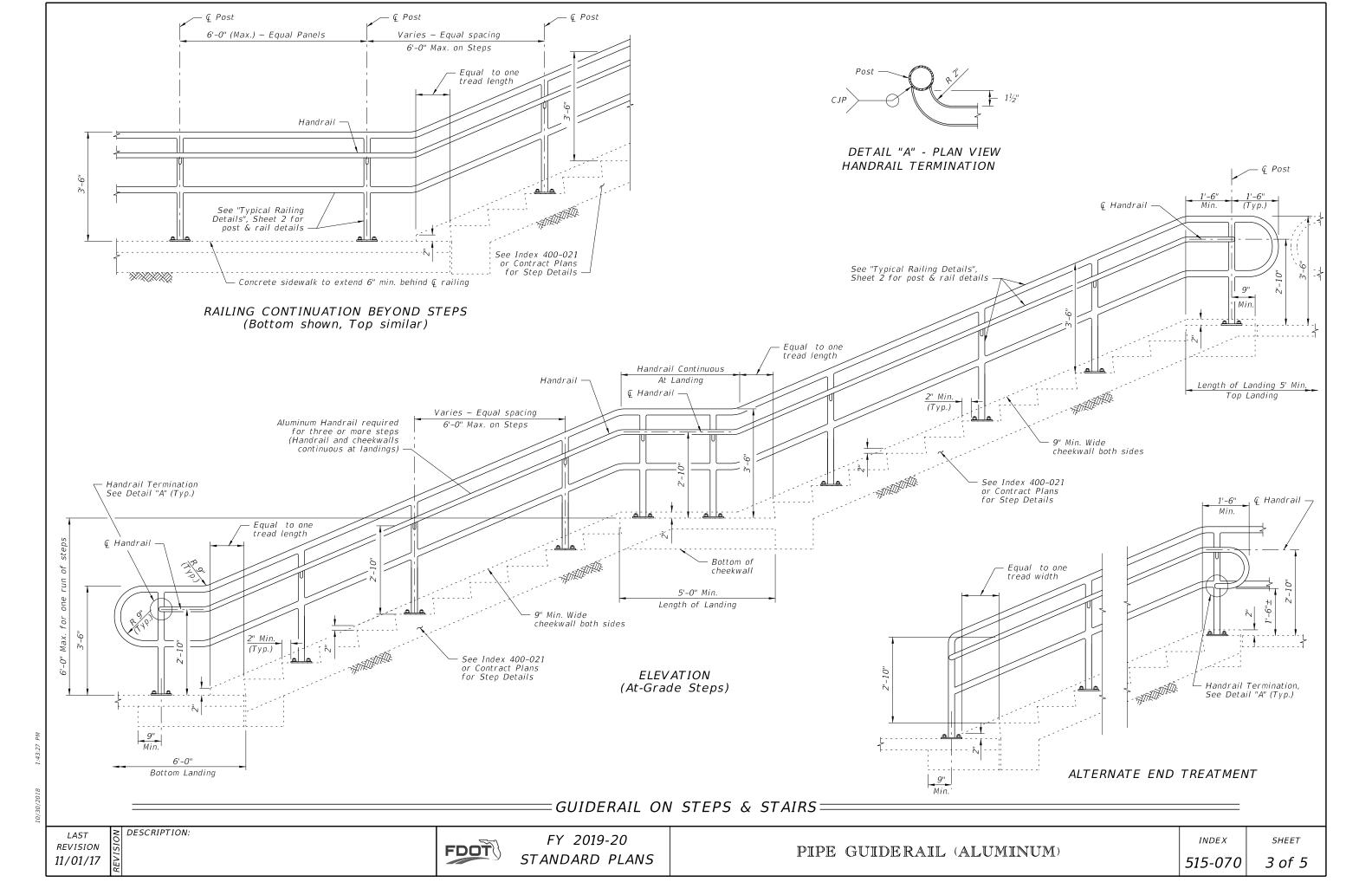
  E. For curved longitudinal alignments, shop bend top and bottom rails and handrails to match the alignment radius.

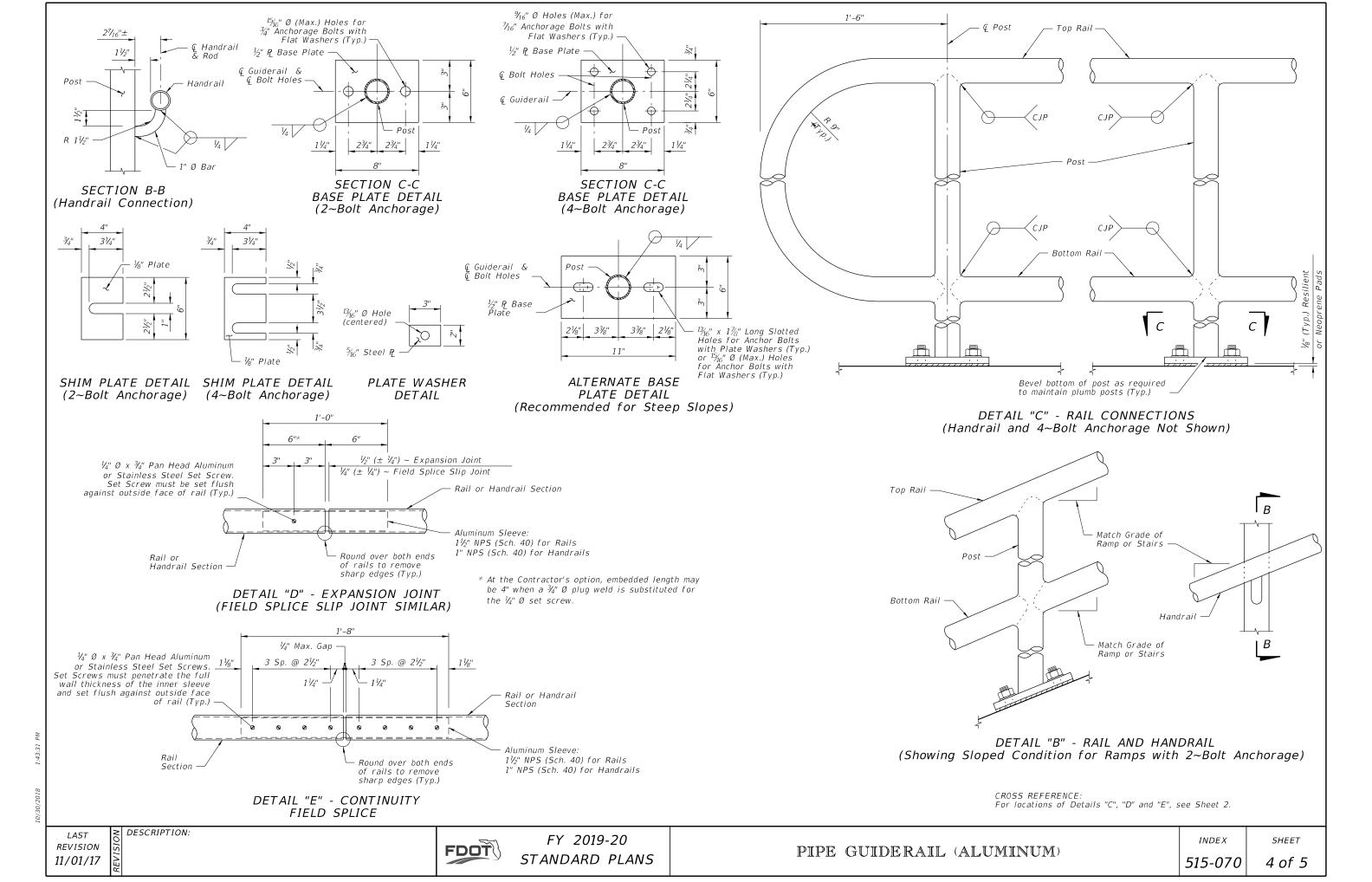
  F. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner, not at the corner apex.
- 5. Handrails are required and must be continuous at landings for:
- A. Grades Steeper than 5%
- B. Three or more steps
- 6. Cutting of reinforcing steel is permitted for post installed anchor bolts.

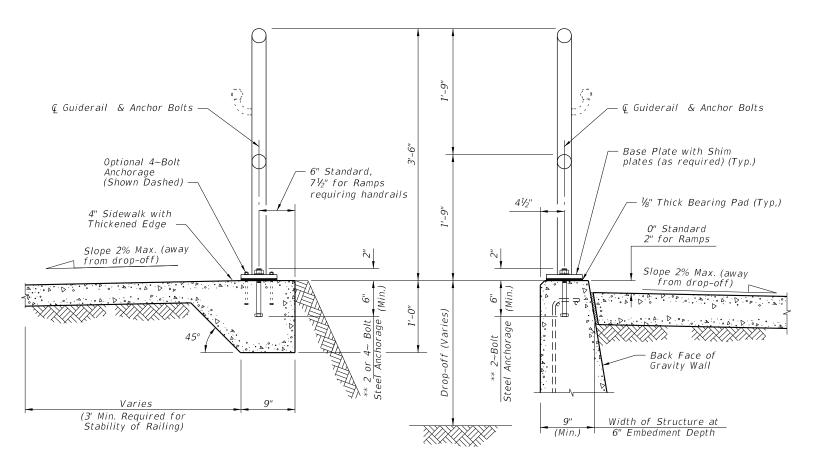
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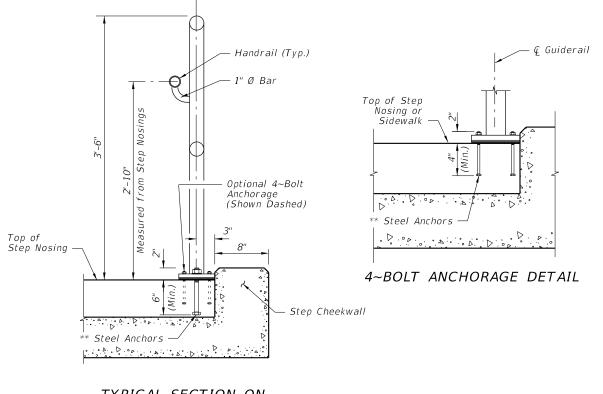












⊊ Guiderail &

Änchor Bolts

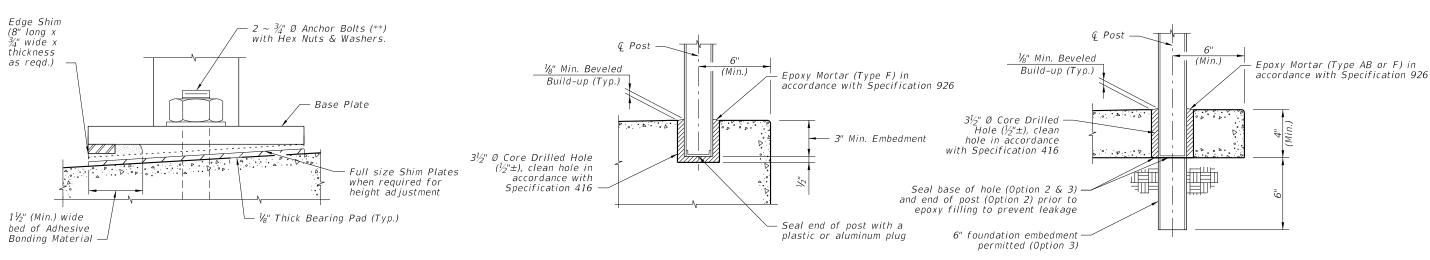
#### TYPICAL SECTION ON CONCRETE SIDEWALK

TYPICAL SECTION ON GRAVITY WALL (Other Retaining Walls Similar)

TYPICAL SECTION ON STEPS & STAIRS

5'-0" Std. ~ 3'-0" Min. Clear |

Between Handrails



DETAIL "F" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

SIDEWALK ANCHORAGE DETAIL OPTION 1

#### SIDEWALK ANCHORAGE DETAIL OPTION 2 & 3

NOTES:

**  $2 \sim \frac{3}{4}$ " Ø x 8" or  $4 \sim \frac{7}{16}$ " Ø x 6" Steel Anchors:

Galvanized Steel Bolts (As Shown) (C-I-P); Galvanized U-Bolts

Permitted (C-I-P); Galvanized Adhesive Anchors Permitted

*** The minimum embedment for Adhesive Anchors is 6" for 2~Bolt Anchorage or 4" for 4~Bolt Anchorage.

**REVISION** 11/01/17

DESCRIPTION:

FDOT

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PIPE GUIDERAIL (ALUMINUM)

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#### NOTES:

- 1. Shop Drawings are required, refer to Specification 515.
- 2. <u>Materials:</u>
- A. Pan Head Set Screws: Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM A36 or ASTM A709 Grade 36
- C. Pipe Rails and Posts: ASTM A53 Grade B for standard weight pipe and ASTM A500 Grade B, C or D or ASTM A501 for Structural Tube.

Handrail Support Bars: ASTM A36

RAILING MEMBER DIMENSIONS TABLE				
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS	
Posts	2" NPS (Sch. 40)	2.375"	0.154"	
Rails	2" NPS (Sch. 40)	2.375"	0.154"	
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) HSS1.500x0.125	1.315" 1.500"	0.133" 0.125"	
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A	

- D. Galvanized Steel Fasteners:
- a. Hex Head Bolts: ASTM A307 Type 1 or ASTM F1554 Grade 36 b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods
- c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- E. Aluminum Shims: ASTM B209, Alloy 6061
- F. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specifications 515 and 932.

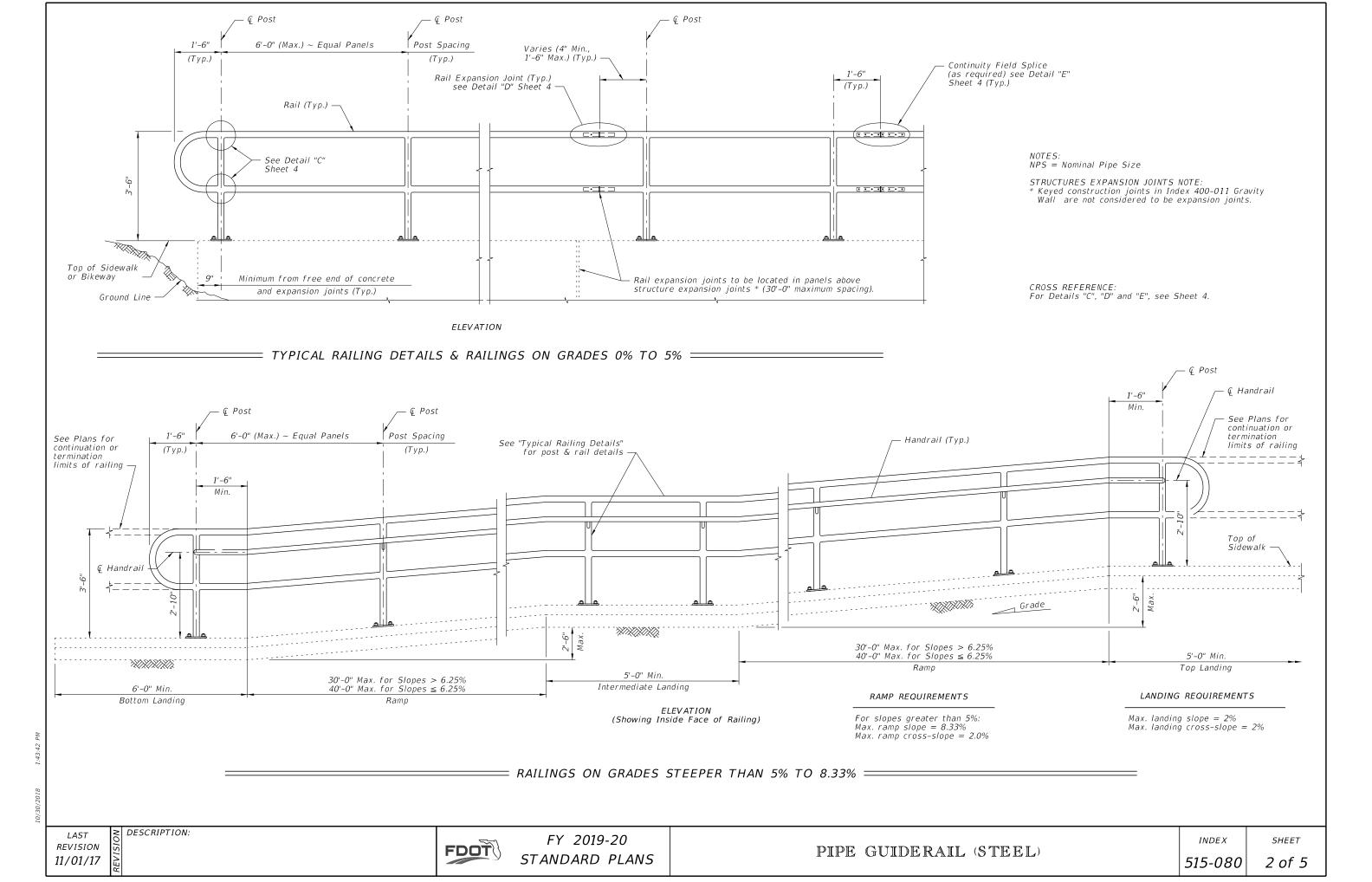
#### 3. Fabrication:

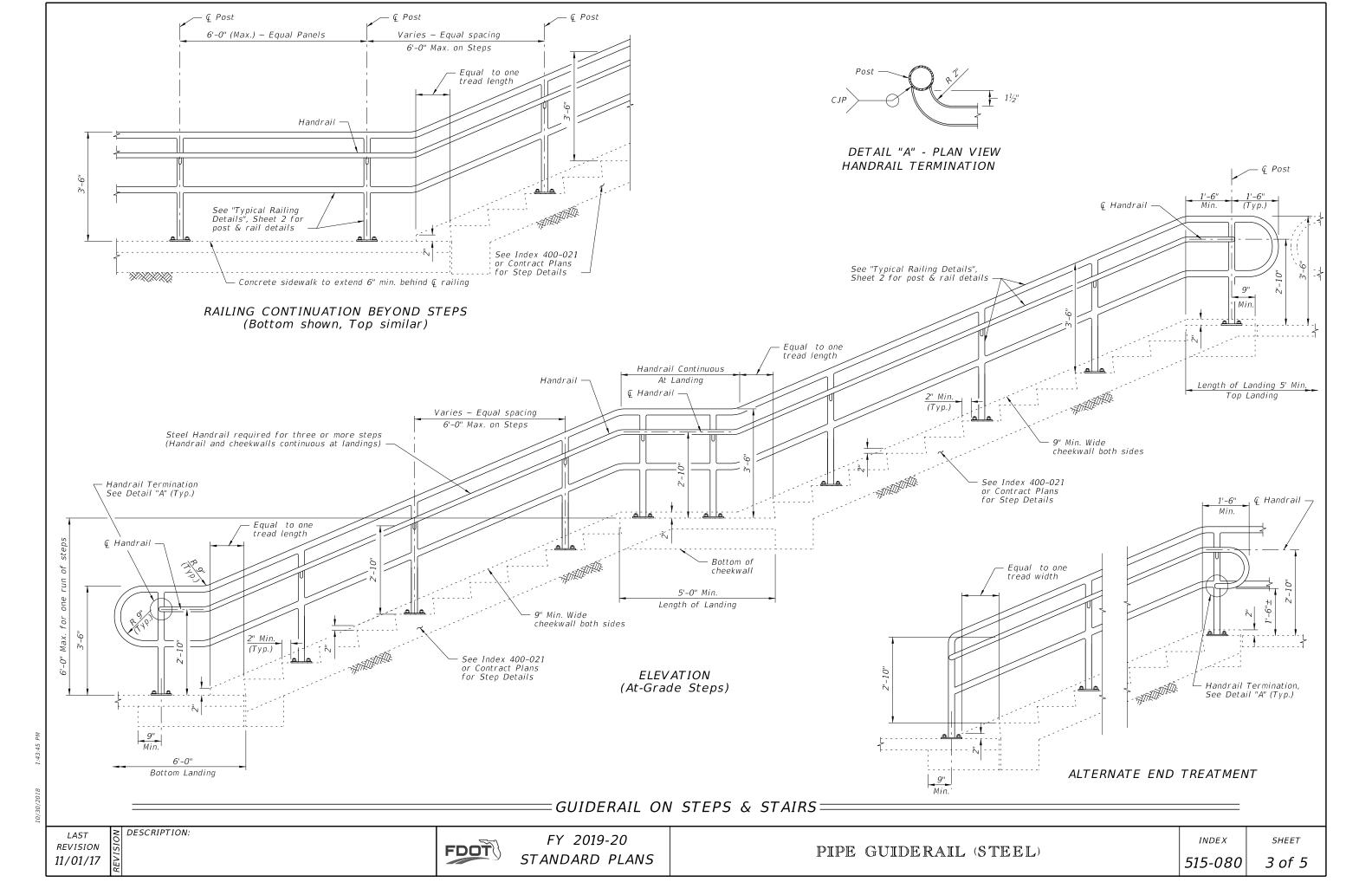
- A. Place expansion joints at a maximum of 30'-0"spacing.
- B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts.
- C. Continuity field splice (Detail "E") only use to make the railing continuous for unforeseen field adjustments
- D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9"bend radius or terminated at adjoining sections with a standard end hoop when handrails are not required.
- E. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- F. For changes in tangential longitudinal alignment greater than 45°, positioned posts a maximum of 2'-0" each side of the corner, not at the corner apex.

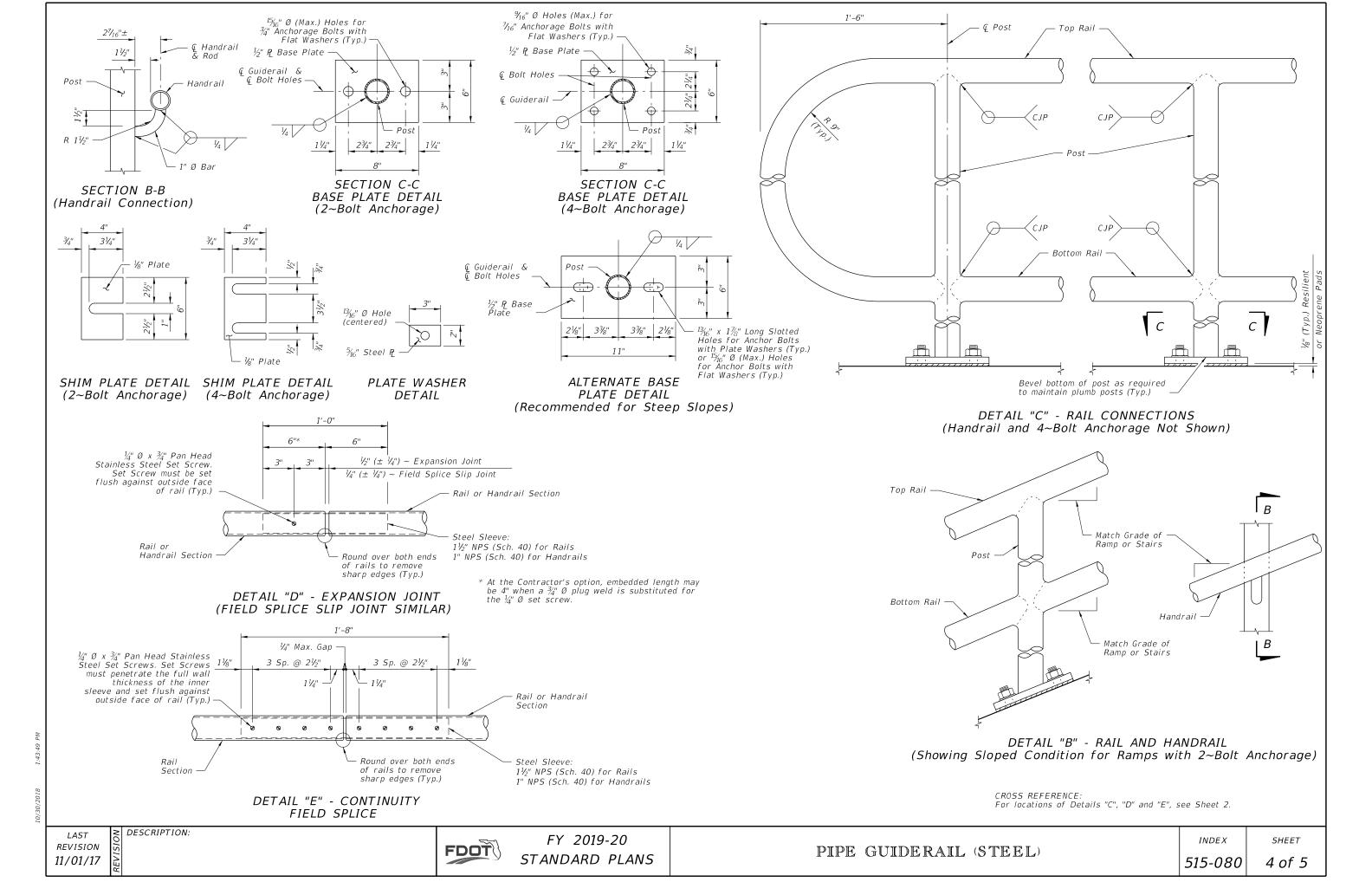
#### 4. Handrails are required and must be continuous at landings for:

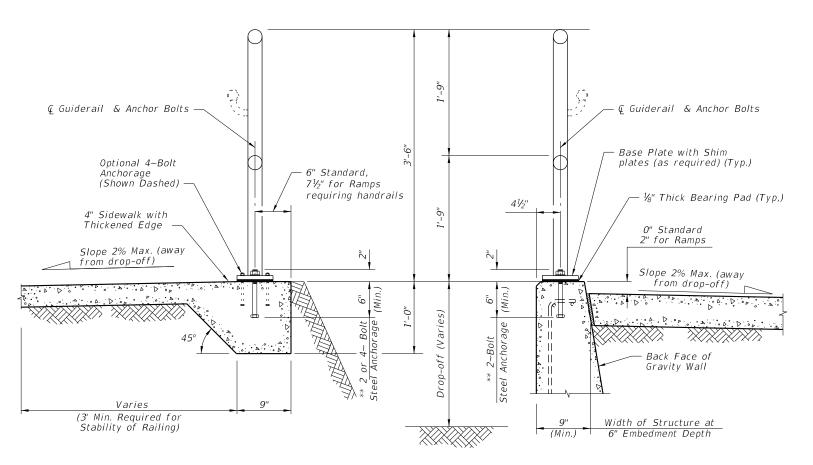
- A. Grades Steeper than 5%,
- B. Three or more steps
- 5. Cutting of reinforcing steel is permitted for adhesive anchor bolt installations.

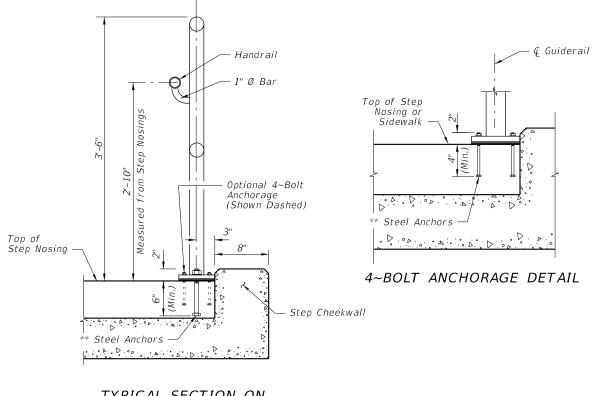
DESCRIPTION:











Guiderail &

Änchor Bolts

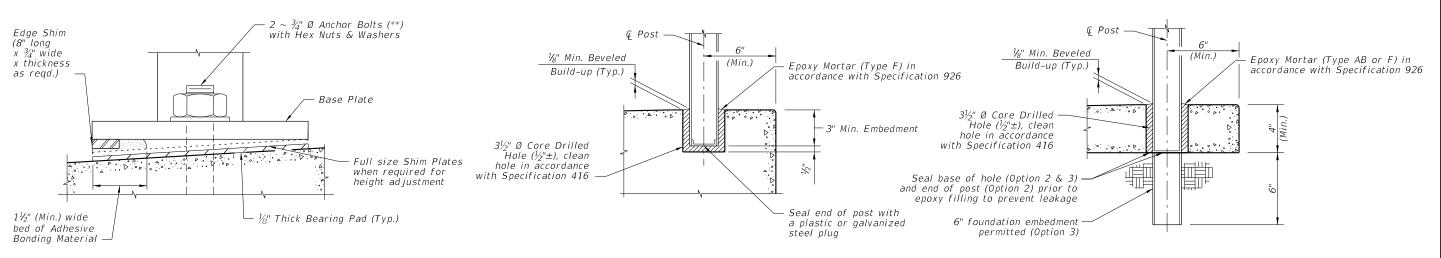
#### TYPICAL SECTION ON CONCRETE SIDEWALK

TYPICAL SECTION ON GRAVITY WALL (Other Retaining Walls Similar)

TYPICAL SECTION ON STEPS & STAIRS

5'-0" Std. ~ 3'-0" Min. Clear |

Between Handrails



DETAIL "F" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

OPTIONAL SIDEWALK ANCHORAGE DETAIL

#### SIDEWALK ANCHORAGE DETAIL OPTION 2 & 3

**2  $\sim \frac{3}{4}$ " Ø x 8" or 4  $\sim \frac{7}{16}$ " Ø x 6" Steel Anchors: Galvanized Steel Bolts (As Shown) (C-I-P); Galvanized U-Bolts Permitted (C-I-P); Galvanized Adhesive Anchors Permitted (***); Expansion Anchors Not Permitted.

*** The minimum embedment for adhesive anchors is 6" for 2~Bolt Anchorage or 4" for 4~Bolt Anchorage.

**REVISION** 11/01/17

DESCRIPTION:

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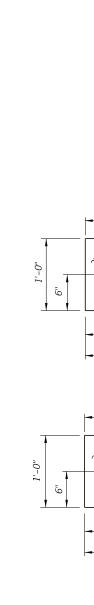
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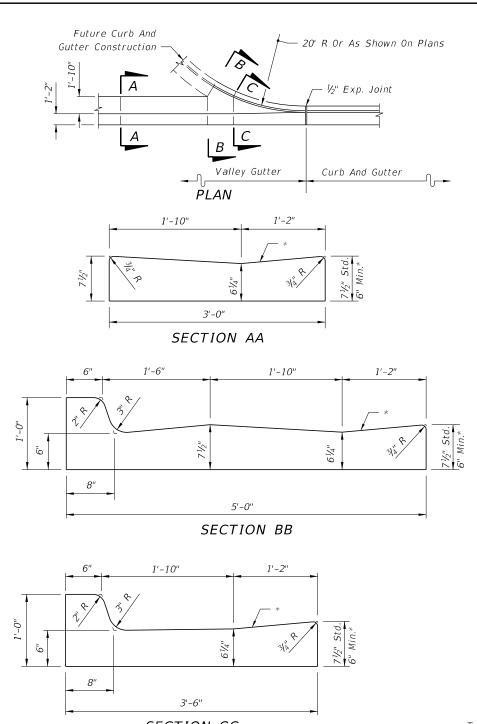
PIPE GUIDERAIL (STEEL)

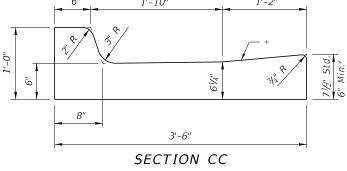
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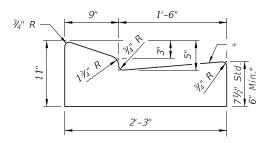


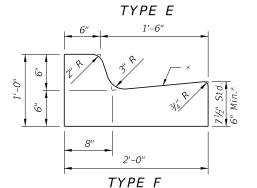


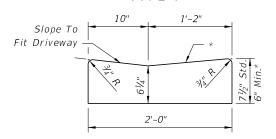
#### VALLEY GUTTER

- * When used on high side of roadways, the cross slope of the gutter shall match the cross slope of the adjacent pavement. The thickness of the lip shall be 6", unless otherwise shown on plans.
- ☑ Rotate entire section so that gutter cross slope matches slope of adjacent circulating roadway pavement.

For use adjacent to concrete or flexible pavement. For details depicting usage adjacent to flexible pavement, see Sheet 2. Expansion joint, preformed joint filler and joint seal are required between curb & gutter and concrete pavement only, see Sheet 2.

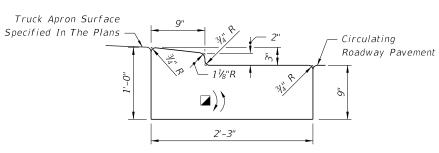




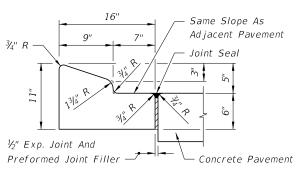


Note: To be paid for as parent curb.

# DROP CURB - Standard Shoulder Line Shoulder Pavement Earth Berm 3'-6" SHOULDER GUTTER

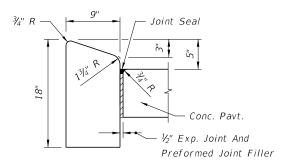


TRAFFIC BEARING SECTION FOR USE IN ROUNDABOUT CENTRAL ISLAND CONSTRUCTION TYPE RA

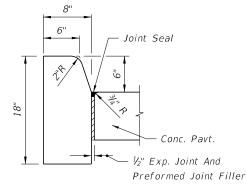


For details depicting usage adjacent to flexible pavement, see Sheet 2.

#### TYPE A



TYPE B



TYPE D

Note: For use adjacent to concrete or flexible pavement, concrete shown. Expansion joint, preformed joint filler and joint seal are required between curbs and concrete pavement only, see Sheet 2.

#### CONCRETE CURB

#### CONCRETE CURB AND GUTTER

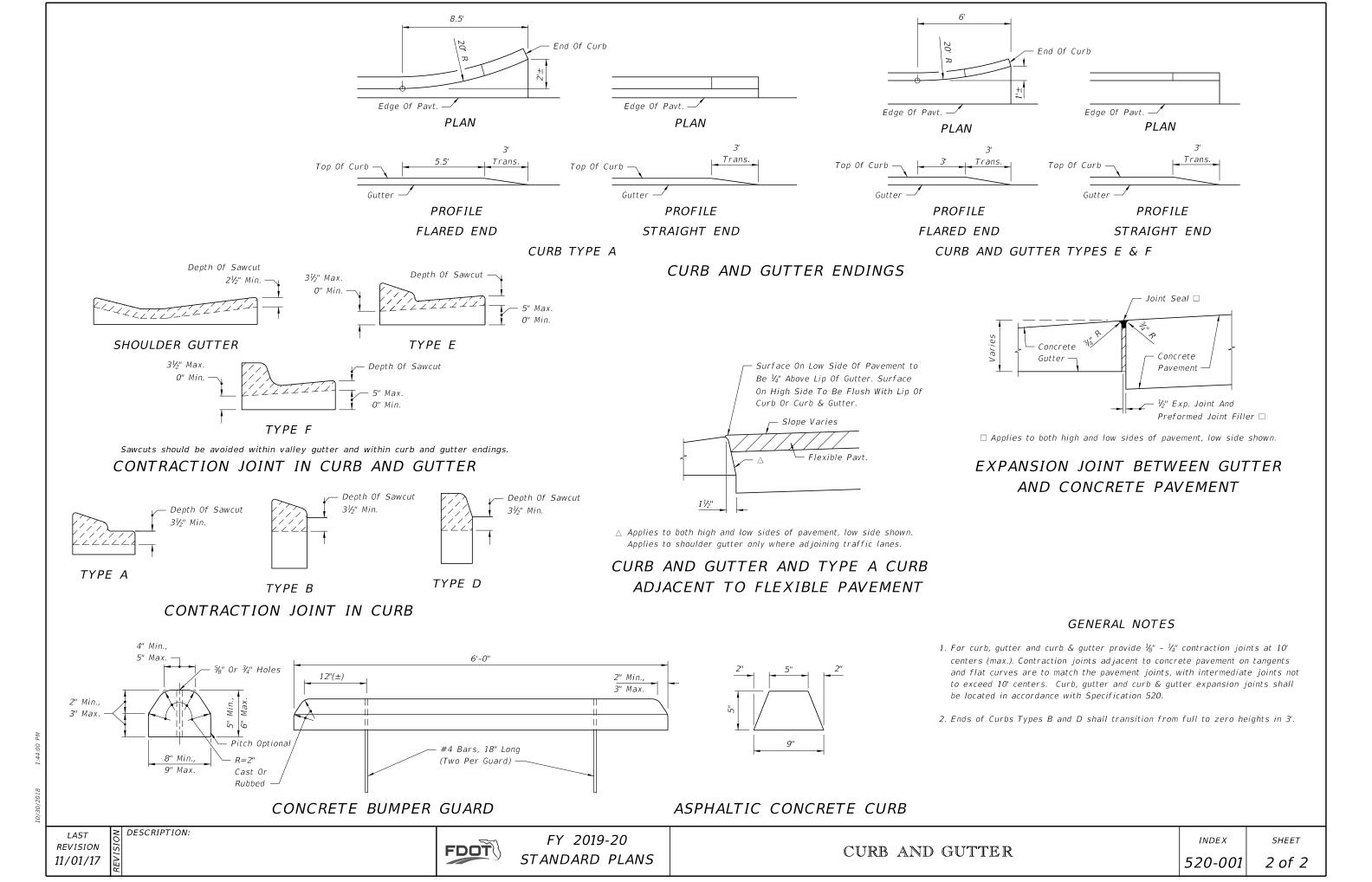
**REVISION** 11/01/17

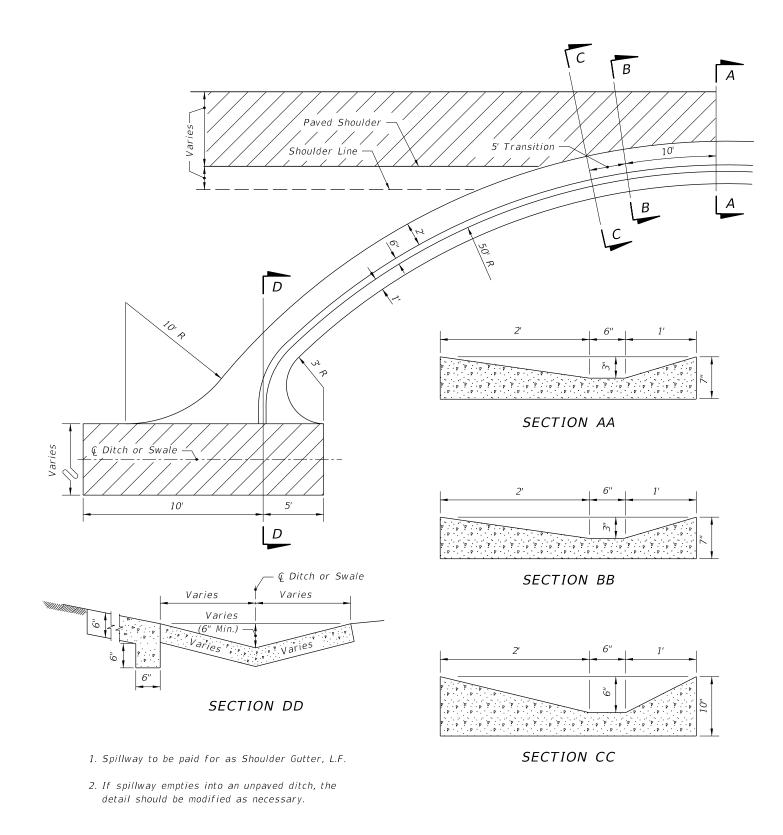
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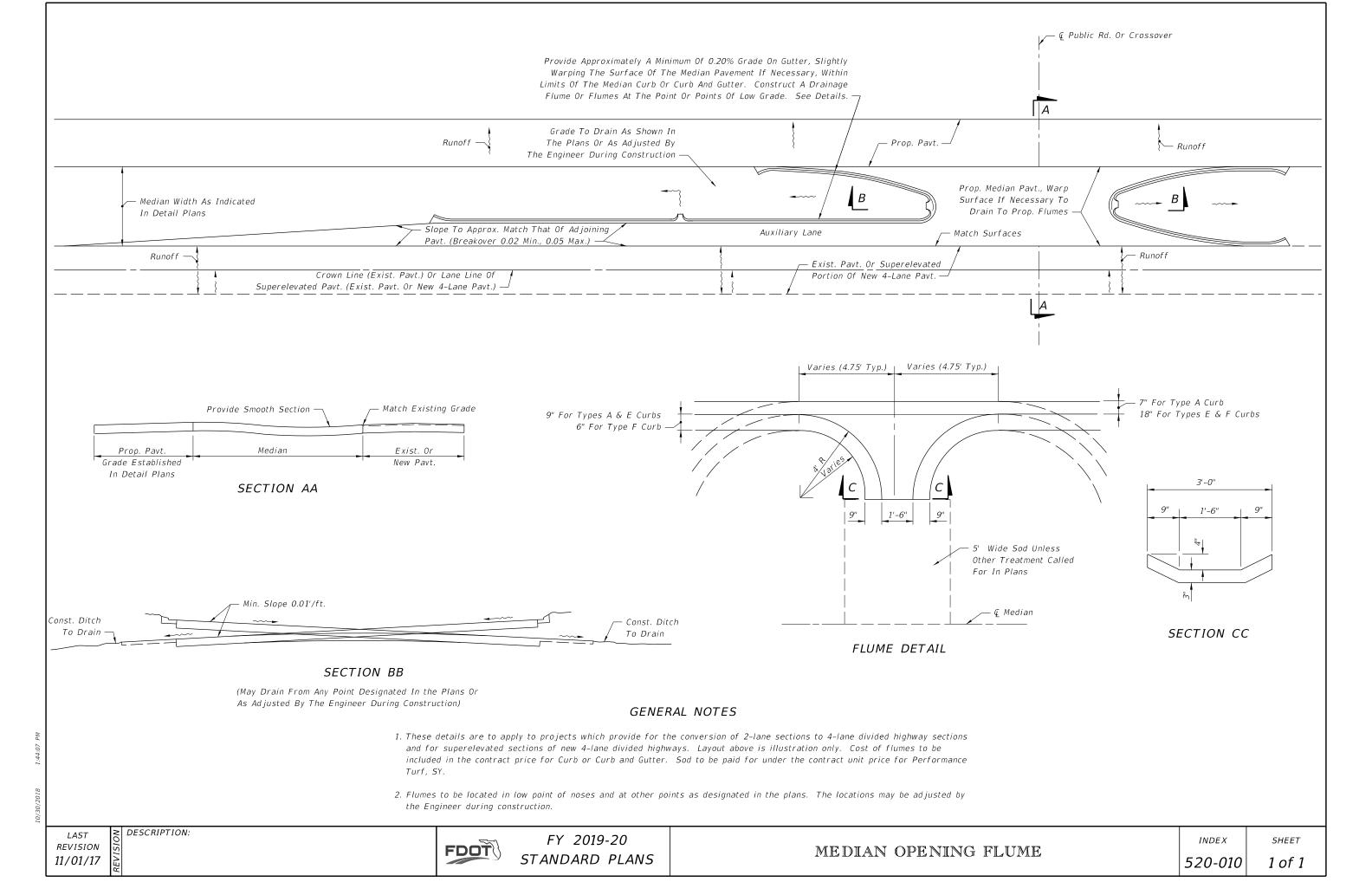


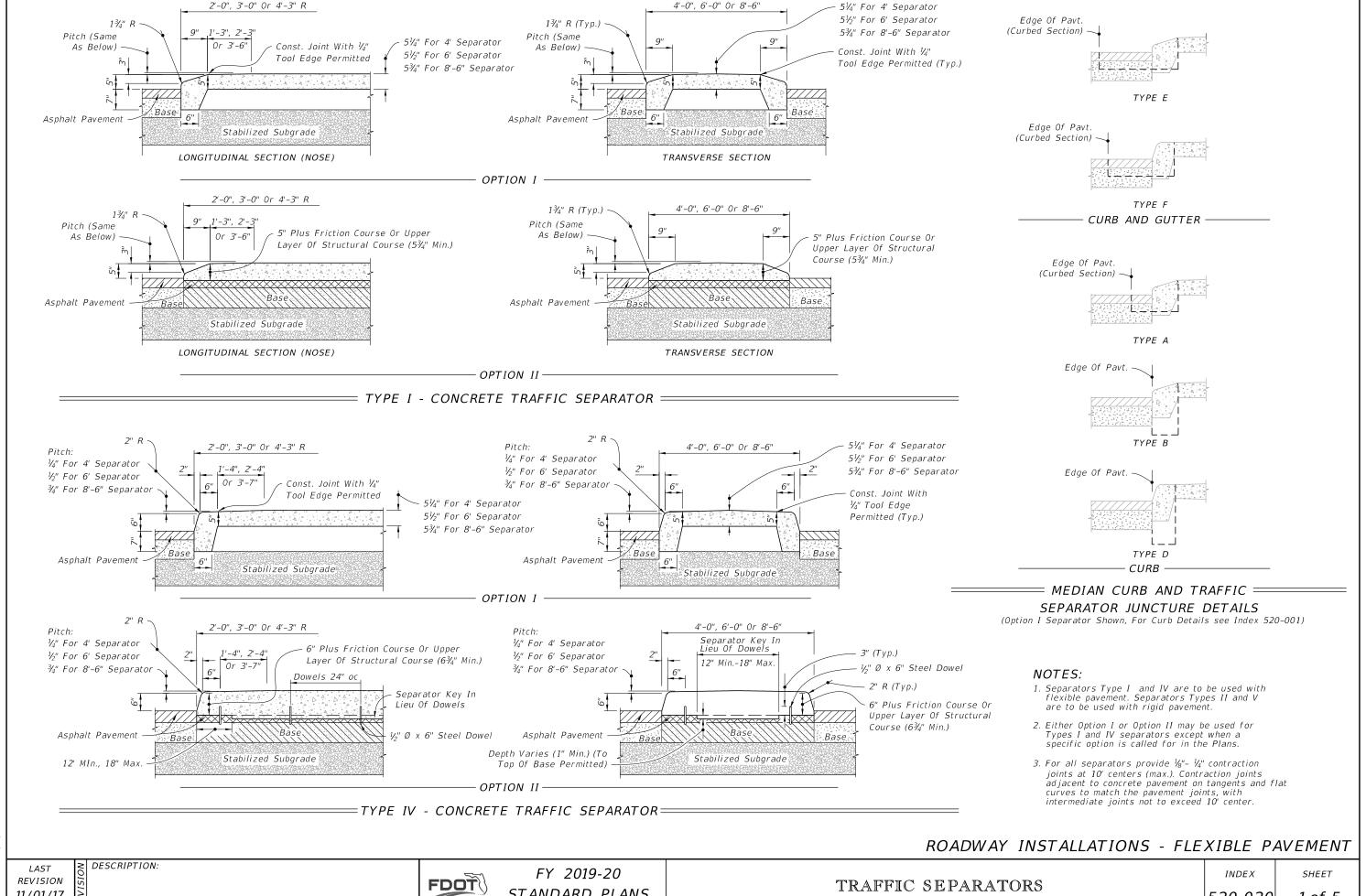
DETAIL OF CONCRETE SPILLWAY AT END OF SHOULDER GUTTER (TO BE USED WHERE INLETS, PIPES & ENDWALLS ARE IMPRACTICAL)

REVISION 11/01/17

≥ DESCRIPTION:

FDOT



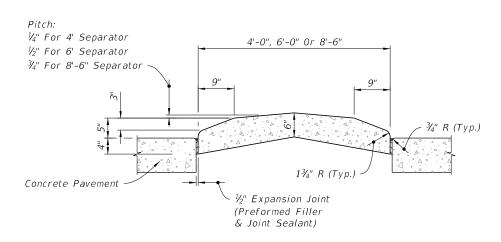


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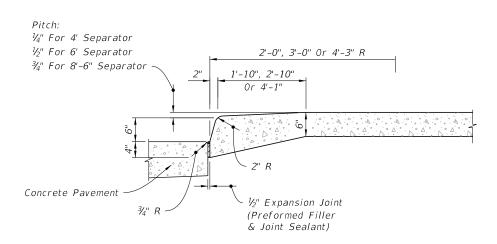
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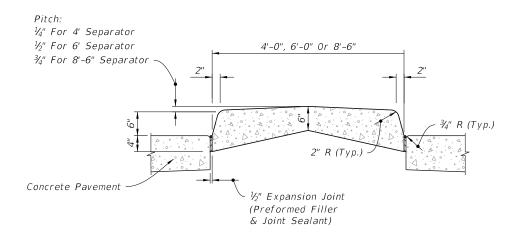


#### LONGITUDINAL SECTION (NOSE)

TRANSVERSE SECTION

= TYPE II - CONCRETE TRAFFIC SEPARATOR =





LONGITUDINAL SECTION (NOSE)

TRANSVERSE SECTION

= TYPE V - CONCRETE TRAFFIC SEPARATOR =

ROADWAY INSTALLATIONS - RIGID PAVEMENT

REVISION 11/01/17

DESCRIPTION:

FDOT

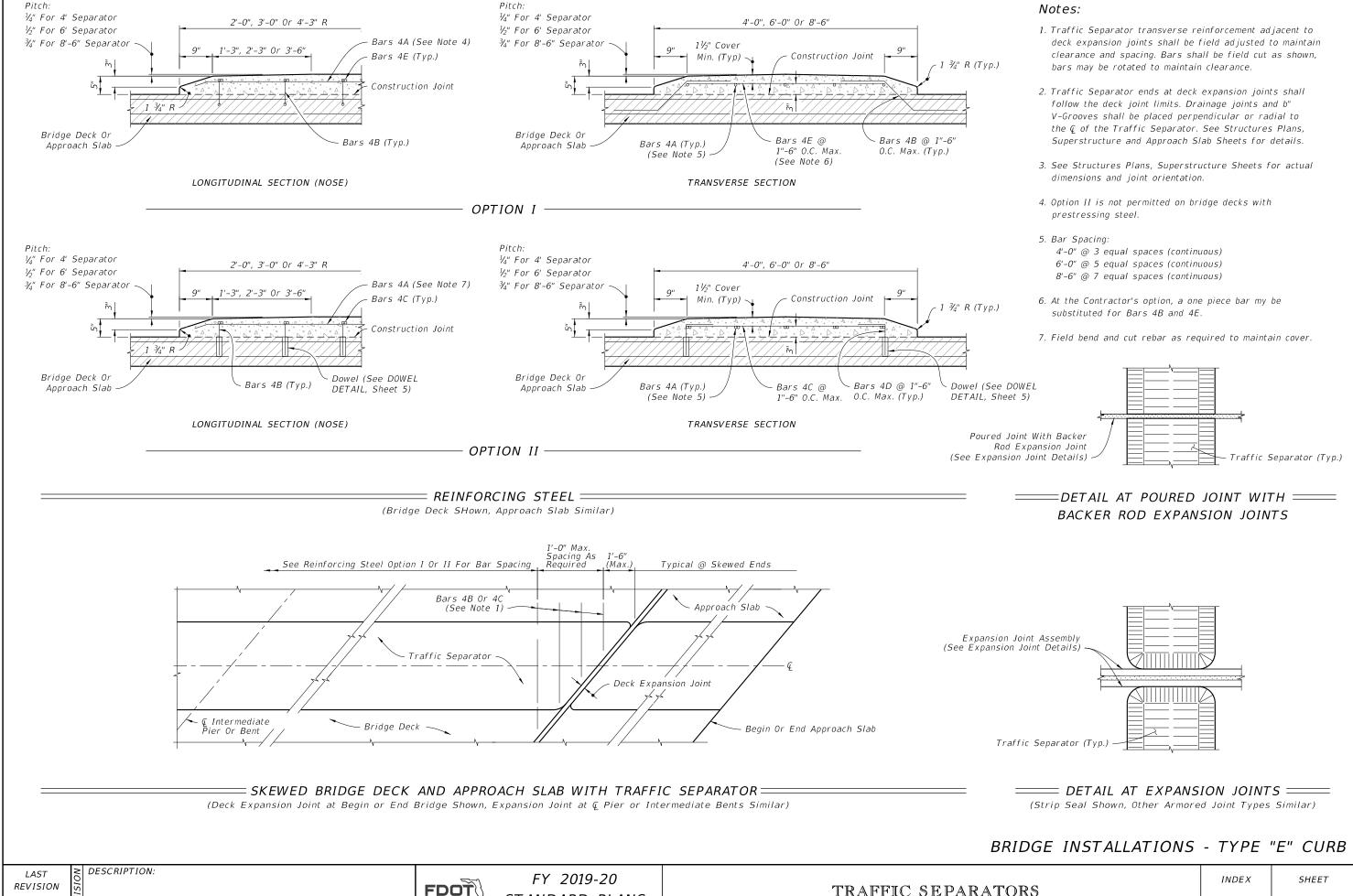
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TRAFFIC SEPARATORS

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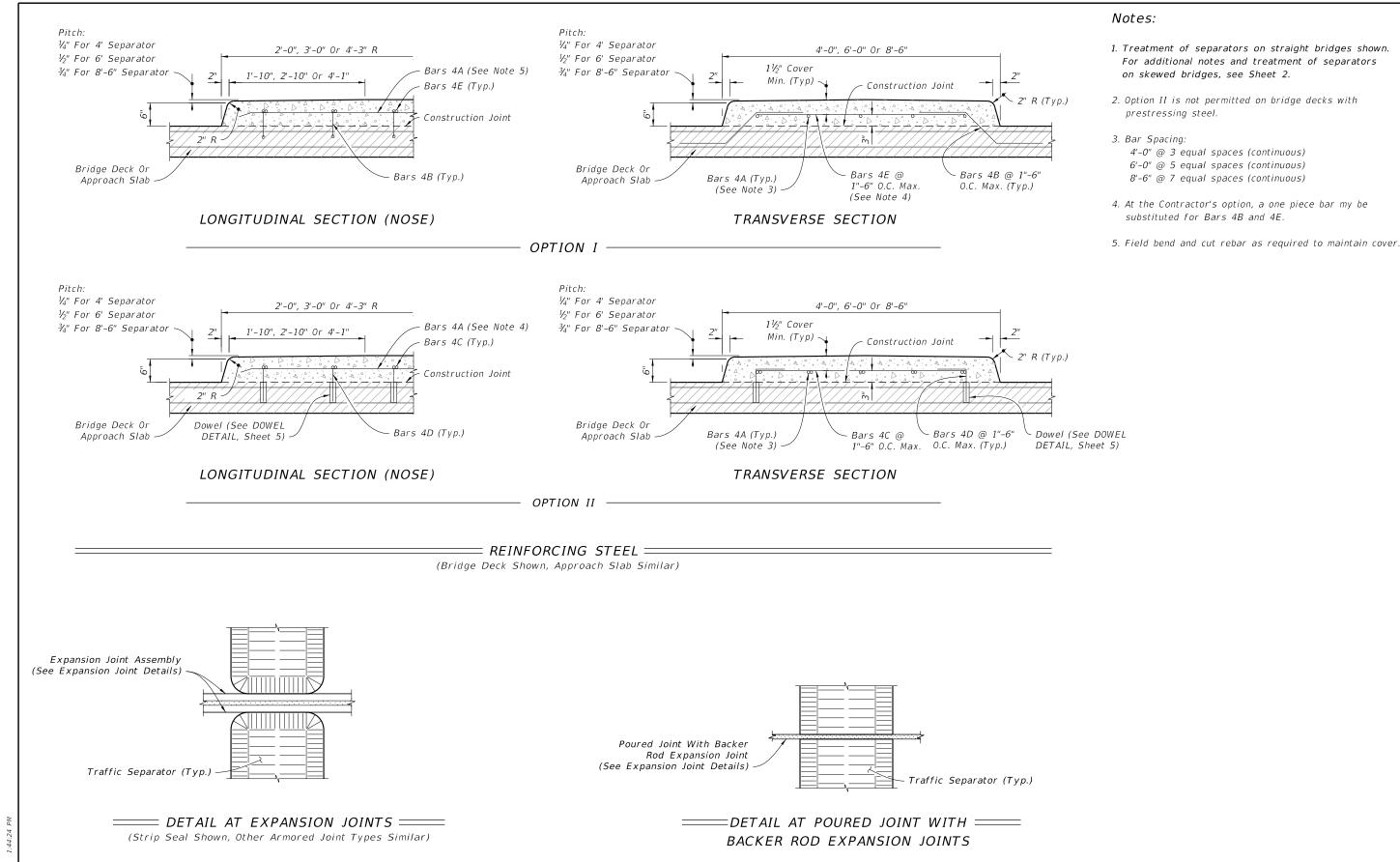
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STANDARD PLANS

TRAFFIC SEPARATORS

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BRIDGE INSTALLATIONS - TYPE "F" CURB

**REVISION** 11/01/17

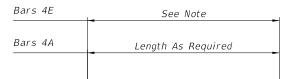
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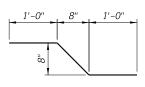
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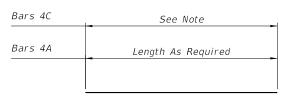


Bars 4A & 4E

Bar 4B

Length of Bars 4E is 2'-5" for 4'-0" Separator. Length of Bars 4E is 4'-5" for 6'-0" Separator. Length of Bars 4E is 6'-11" for 8'-6" Separator.

#### — OPTION I —





Bars 4A & 4C

Bar 4D

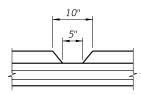
Length of Bars 4C is  $2'-4\frac{1}{2}"$  for 4'-0" Separator. Length of Bars 4C is  $4'-4\frac{1}{2}''$  for 6'-0'' Separator. Length of Bars 4C is  $6'-10\frac{1}{2}''$  for 8'-6'' Separator.

#### — OPTION II —

#### REINFORCING STEEL NOTES:

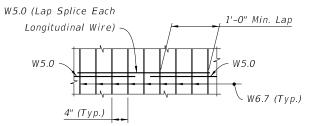
- 1. All dimensions are out to out.
- 2. The 8" vertical dimension shown for Bars 4B and 4D are based on a slab  $8\frac{1}{2}$ " thick or greater without a wearing surface. If slab thickness is less than 81/3", decrease this dimension by an amount equal to the difference in thickness. If a wearing surface is to be provided, increase this dimension by an amount equal to the wearing surface thickness.

#### CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS



See Structures Plans, Superstructure Sheets for location(s) of drainage joints. Locations for drainage joints shall be limited to the constant width section of separator.

> = DRAINAGE JOINT DETAIL = (For 5" Opening Or Less)



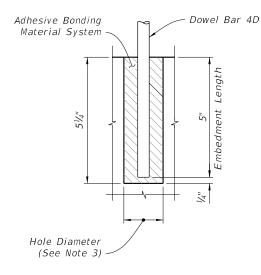
#### SPLICE DETAIL (Between WWR 3 x 4 - W5.0 x W6.7 Sections)

OPTION A: Use Welded Wire Reinforcement 3 x 4 - W5.0 x W6.7 as required by plans in place of Bars 4A, 4B and 4E. Bend the Welded Wire Reinforcement to the dimensions of Bar 4B shown in the Bending Diagram for Reinforcing Steel Option I.

OPTION B: Use Welded Wire Reinforcement 3 x 4 - W5.0 x W6.7 as required by plans in place of Bars 4A and 4C shown in Reinforcing Steel Option II.

NOTE: Welded Wire Reinforcement to consist of smooth wire meeting the requirements of Specification 931.

#### === ALTERNATE REINFORCING STEEL DETAILS====== (Welded Wire Reinforcement)



#### DOWEL NOTES:

- 1. Shift Dowel Holes to clear if existing reinforcement is encountered.
- 2. Provide and install an adhesive bonding material system in accordance with Specifications 416 and 937.
- 3. The dowel hole diameter is to meet adhesive bonding material system manufacturer's requirements.

#### DOWEL DETAIL

#### ESTIMATED TRAFFIC SEPARATOR QUANTITIES:

CONSTANT WIDTH OF SEPARATOR:

	TYPE "E"	TYPE "F"
4'-0"	Width = 0.056 CY per Ft.	- 0.072 CY per F
6'-0"	Width = 0.089 CY per Ft.	- 0.112 CY per Fi
8'-6"	Width = 0.132 CY per Ft.	- 0.164 CY per Fi

#### NOSE:

	<i>TYPE "E"</i>		TYPE "F
4'-0"	Width = 0.080 CY	-	0.109 CY
6'-0"	Width = 0.193 CY	-	0.257 CY
8'-6"	Width = 0.403 CY	_	0.536 CY

#### REINFORCING STEEL:

(All quantities are based on an  $8\frac{1}{2}$ " slab.)

#### OPTION I:

4'-0" Width - 6.37 Lbs. per Ft. 6'-0" Width - 8.60 Lbs. per Ft. 8'-6" Width - 11.05 Lbs. per Ft.

#### OPTION II:

4'-0" Width - 4.77 Lbs. per Ft. 6'-0" Width - 7.00 Lbs. per Ft. 8'-6" Width - 9.45 Lbs. per Ft.

BRIDGE INSTALLATIONS - TYPE "E" AND "F" CURB

**REVISION** 11/01/17

DESCRIPTION:

FY 2019-20 STANDARD PLANS

TRAFFIC SEPARATORS

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4	Median Barrier - Sloped End Treatment
5	Median Barrier - Grade Separated
6	Median Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding - Symmetrical
7	Median Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding - Asymmetrical
8	Median Barrier - 56" Height Section for Barrier-Mounted Dual Sign Support Shielding - Min. Width
9	Median Barrier - 38" Height Split Section for Stand-Alone Sign Support Shielding
10	Median Barrier - 44" Height Split Section for Pier Shielding
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23	Wall Shielding Barrier – 38" Height Section – Approach and Trailing Transition
24	Wall Shielding Barrier – 38" Height Section – Guardrail Connection
25	Wall Shielding Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding
26	Reinforcing Bar Bending Diagrams

#### **GENERAL NOTES:**

SHEET CONTENTS

- 1. BARRIER CONCRETE: Use Class II concrete for all barriers constructed in slightly aggressive environments, and use Class IV Concrete for all barriers constructed in moderately or extremely aggressive environments. On all exposed surfaces, apply a Class 3 surface finish in accordance with Specification 400.
- 2. STEEL BAR REINFORCEMENT: Where required to maintain continuity, provide lap splices of at least 18 inches for No. 4 bars and 20 inches for No. 5 bars, unless otherwise shown herein (including shorter splices as provided by the default bar bending diagrams).

The default reinforcing details shown herein, including bar shapes and lap splice positions, are intended to show required steel locations and provide for a constructible design. However, with the approval of the Engineer, alternate steel configurations may be used in the same locations shown herein, given that the equivalent strength reinforcing is provided and the cover, maximum spacing, and continuity requirements are maintained.

3. OPTIONAL WELDED WIRE REINFORCEMENT: With the approval of the Engineer, steel welded wire reinforcement in accordance with Specification 415 may be substituted for the steel bars shown herein. Place the welded wire in the same locations specified for the steel bars, and maintain the equivalent strength, cover, maximum spacing, and continuity requirements.

#### GENERAL NOTES (CONTINUED):

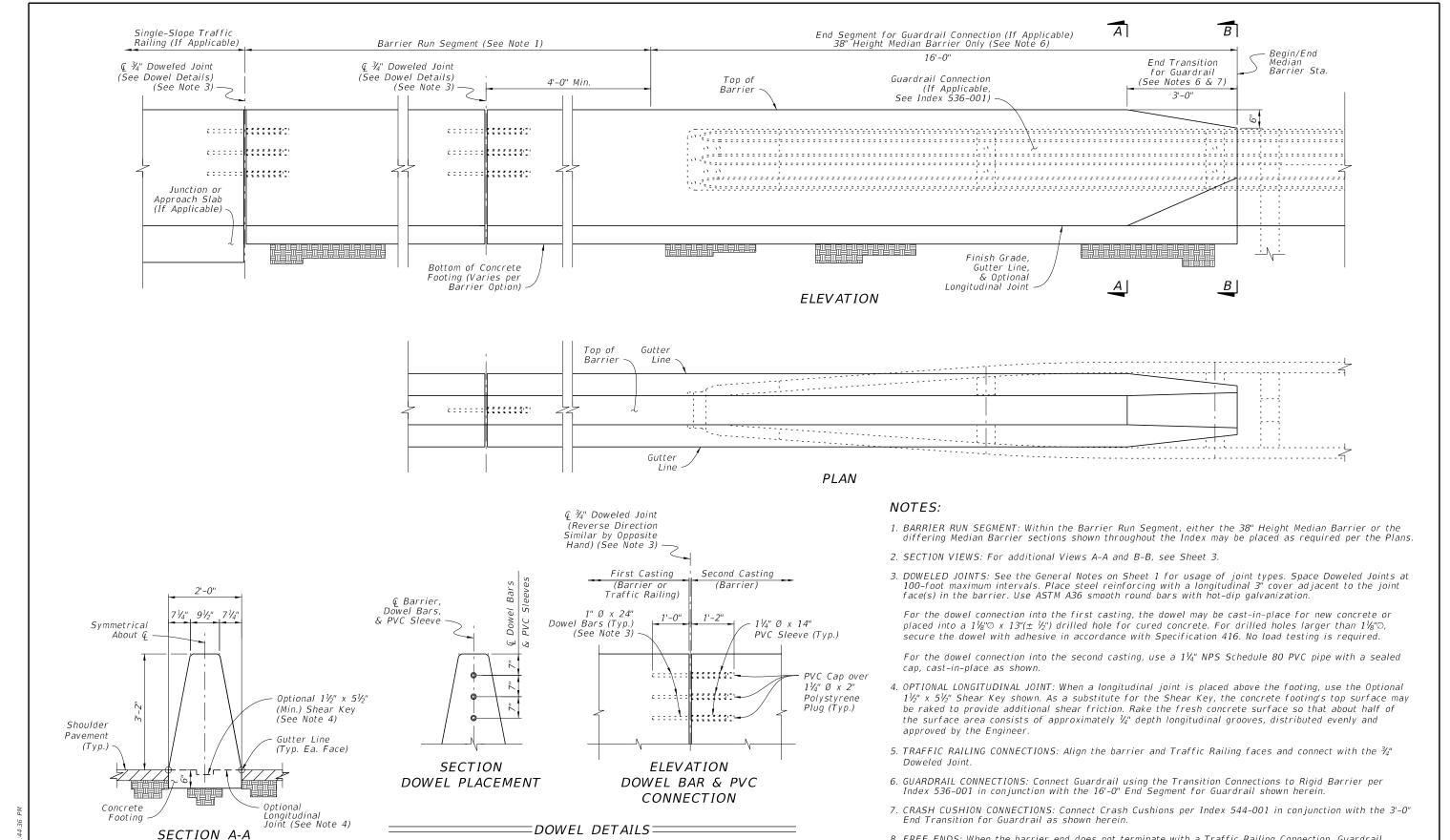
- 4. TOP FACE LONGITUDINAL REINFORCEMENT: Unless otherwise specified, the longitudinal reinforcement shown closest to the top face of the barrier has a maximum cover of  $4\frac{1}{2}$ , measured from the top face of the barrier.
- 5. MINIMUM BARRIER LENGTH: Unless otherwise shown in the Plans, the minimum Concrete Barrier length is 40 feet.
- 6. CONSTRUCTION JOINTS: Install Construction Joints only as needed for discontinuous concrete casting or cold joints. Maintain continuity of steel reinforcement across Construction Joints. Construction Joints are classified herein as Transverse Joints or Longitudinal Joints.

Transverse Joints are permitted at 20-foot or greater intervals along the barrier. For Tall Grade-Separated Sections, see Sheet 5 for additional Transverse Joint requirements.

Longitudinal Joints are only permitted where indicated in the following details and notes, with a vertical position tolerance of  $\pm 1\frac{1}{2}$ " from the locations shown.

- 7. DOWELED JOINTS: As shown in the Dowel Details on Sheets 2 & 13, install 3/4" Doweled Joints for Concrete Barrier connections to Pier Protection Barrier and Traffic Railings. Doweled Joints are also required for expansion mitigation in Median Barrier as defined per Sheets 2 & 5. Doweled Joints are not permitted within Grade-Separated Median Barrier.
- 8. CRACK CONTROL V-GROOVES: At 20-foot intervals, place  $\frac{3}{8}$ " depth V-grooves that run vertically and/or transversely in the front, top, and back faces of barriers. The V-grooves can be either molded or scored while the concrete is still plastic.
- 9. SUBGRADE: Compact the top layer of subgrade with Type B Stabilization, LBR 40 (12 in.).
- 10. FOOTING BOTTOM CONCRETE COVER: At the bottom of barrier footings shown throughout this Index, up to 2 inches of additional concrete cover is permitted beyond what is shown herein to accommodate soil grade irregularities.
- 11. FINISH GRADE ELEVATION: At the barrier face location, the finish grade pavement has a vertical position tolerance of ± ½" from the locations shown herein, relative to the barrier elevation. Maintain visually smooth and even pavement at the barrier face, per the approval of the Engineer.
- 12. DRAINAGE INLETS: Where called for in the Plans, install corresponding inlets per Indexes 425-030 thru 425-032.
- 13. LIGHT POLE MOUNTING: Where called for in the Plans, install aluminum light poles per Index 715-002.
- 14. OPAQUE VISUAL BARRIER: Where called for in the Plans, install Opaque Visual Barrier per Index 521-010.
- 15. BARRIER END MARKERS: For all free ends of concrete barriers that are not shielded with an end treatment or connection to another barrier or traffic railing type, install a Type 3 Object Marker on the end face per Specification 705.
- 16. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification 705. For median barriers, mount the delineator on the top of the barrier, at the centerline of barrier, with reflective sheeting facing traffic on both approaches. For shoulder barriers and split sections, mount the delineators on the top of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.
- 17. TOLL SITES: Where called for in the Plans, substitute the steel reinforcing bars shown herein with GFRP reinforcing bars of the same size. Construct GFRP reinforcing bars in accordance with Specification 932, and use a  $4\frac{1}{2}$ " inner diameter for bar bends. Where required to fit pull boxes while maintaining bar spacing and concrete cover, trim GFRP bars as defined in the Plans.

At toll site locations, the use of Median Barriers on outside shoulders is permitted where called for in the Plans. Shoulder Pavement shown herein may be substituted with material for an alternate usage where defined in the Plans.



**REVISION** 11/01/18

38" HEIGHT MEDIAN BARRIER

(See Sheet 3 for Steel Reinforcing Details)

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

CONCRETE BARRIER

MEDIAN BARRIER SHEET

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accordance with the Free End Reinforcing detail on Sheet 3.

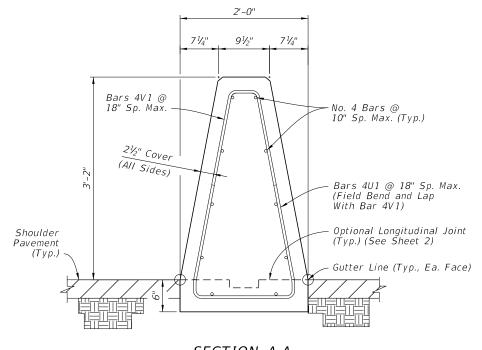
8. FREE ENDS: When the barrier end does not terminate with a Traffic Railing Connection, Guardrail

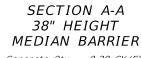
Connection, Crash Cushion Connection, or Sloped End Treatment as called for in the Plans, terminate in

PLAN VIEW - 38" HEIGHT MEDIAN BARRIER FREE END REINFORCING (See Note 3)

## No. 4 Bars (Only Top And Bottom Bars - Field Cut Bars 4V1 & 4U1 Shown For Clarity, $\overline{A}$ Others Similar) 3" Cover B91/2" Bars 4V1 & 4U1 @ 18" Sp. Max. 4 Sp. @ 8" (±½") 3'-0" End Transition for Guardrail

PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (See Note 3)





VIEW B-B REDUCED SECTION OF END TRANSITION FOR GUARDRAIL (End of Barrier)

1'-6"

11¾"

No. 4 Bars Tapered

with Barrier Height

Cover Varies

(Diagonal

Segment)

Shoulder

(Typ.)

Pavement

Bars 4V1

(Field Cut to Fit Vertically as Reqd. & Field Bend to

No. 4 Bars @ 10" Sp. Max.

(Field Cut To Fit

Transversally As

Reqd. & Field

Bend To Lap

With 4V1)

21/2" Cover

Lap with Bars 4U1)

# 2½" Cover Concrete Qty. = 0.20 CY/FT Steel Qty. = 11.8 LB/FT

## 3. PLAN VIEWS: Only top and bottom longitudinal reinforcing is shown for clarity. For all longitudinal steel locations, see the section views.

1. GENERAL: Work with the Plan and Elevation Views

2. BAR BENDING DIAGRAMS: For additional information on Bars 4V1 and 4U1, see the details

NOTES:

on Sheet 2.

on Sheet 26.

## MEDIAN BARRIER - REINFORCING DETAILS

**REVISION** 11/01/18

DESCRIPTION:

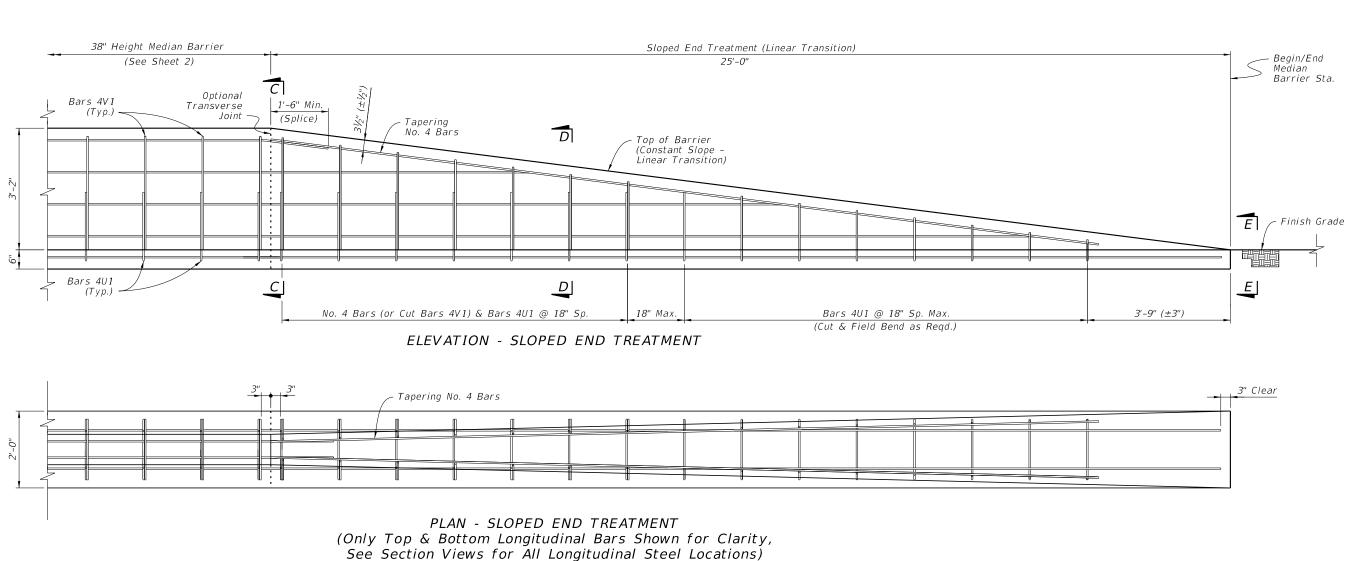
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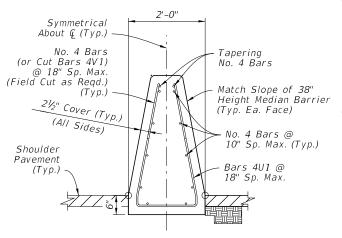
FY 2019-20 STANDARD PLANS

CONCRETE BARRIER

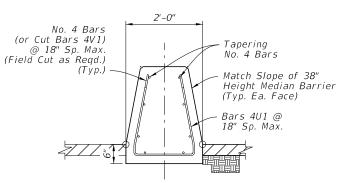
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SECTION C-C **BEGIN TRANSITION** REINFORCING (Height Varies Linearly per Elevation View)



SECTION D-D INTERMEDIATE TRANSITION REINFORCING (Height Varies Linearly per Elevation View)

VIEW E-E **END TRANSITION** 

#### NOTES:

- 1. GENERAL: Install Sloped End Treatment only where called for in the plans.
- 2. JOINTS: Construction or Doweled Joints are not permitted within the Sloped End Treatment segment.

MEDIAN BARRIER -SLOPED END TREATMENT

**REVISION** 

DESCRIPTION:

FDOT

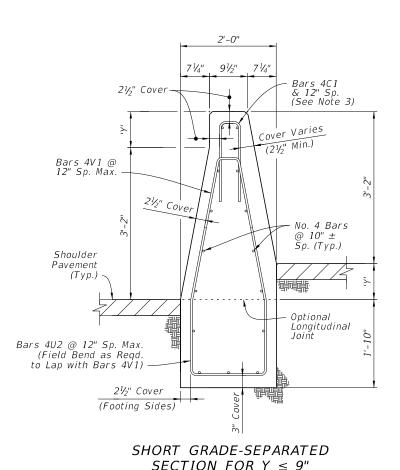
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CONCRETE BARRIER

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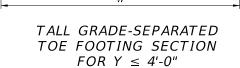
### 91/2" 71/4" No. 4 Bars @ 8" Sp. Max. (Typ.) 2" Cover (Barrier & Stem) Shoulder Pavement of Optional No. 5 Bars Longitudinal @ 8" Sp. (See Note 4) Shoulder Pavement Lower Limit of Optional Longitudinal Joint(s) No. 5 Bars 2" Cover 3" Cover @ 8" Sp. Max. Hook Hook (Length = 'W'-5")(Min.) (Min.)

TALL GRADE-SEPARATED

HEEL FOOTING SECTION

 $FOR Y \leq 4'-0''$ 

2'-0"



Hook

(Min.)

No. 5 Bars

@ 8" Sp. Max.

(Length' = 'W'-5")

2'-0"

71/4" 91/2" 71/4"

2" Cover

(Barrier

& Stem)

No. 5 Bars

(See Note 4)

Shoulder

Pavement

3" Cover

@ 8" Sp.

No. 4 Bars @ 8"

Shoulder

Pavement

Sp. Max. (Typ.)

Upper Limit

of Optional

Joint(s)

Longitudinal

Lower Limit

of Optional

Joint(s)

2" Cover

Hook

(Min.)

Longitudinal

#### NOTES:

- 1. GENERAL: Install the Grade-Separated sections where shown in the Plans and as required to accommodate vertical offsets in pavement of Height Y. Doweled Joints are not permitted within Grade-Separated sections.
- 2. CONNECTIONS BETWEEN DIFFERENT SECTIONS: Connect Short Grade-Separated sections and Tall Grade-Separated sections using a continuous pour or Transverse Joint, where longitudinal steel that aligns within the adjacent section is maintained continuously between sections or has a full lap splice with the adjacent section's longitudinal steel. Connect Short Grade-Separated sections and 38" Height Median Barrier sections of Sheet 2 using a ¾" Doweled Joint.
- 3. SHORT GRADE-SEPARATED SECTIONS: Bars 4C1 and the two uppermost longitudinal bars may be omitted for segments where Y < 2".
- 4. TALL GRADE-SEPARATED SECTIONS: For the vertical and transverse steel reinforcement shown in the Tall Grade-Separated Sections, bar bending diagrams are not provided due to varying section dimensions and Longitudinal Joint locations. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

Longitudinal Joints are permitted between the vertical limits shown, and must remain level and at a consistent height per each continuous casting of concrete. Longitudinal Joints may change elevations at Transverse Joint locations. Field bending of bars is permitted at Longitudinal Joint locations.

Transverse Joints between Tall Grade-Separated Sections do not require continuous steel across the joint if the following conditions are met:

- i. The barrier length on both sides is at least 40 feet, where each segment has continuous
- ii. The barrier's vertical steel spacing is reduced to 4" O.C. for a total of 12 spaces on both sides of the joint.

Grade separation Heights of  $Y \leq 9$ " are permitted on a limited basis using the Tall Grade–Separated section; this is to accommodate cases where maintaining the spread footing through lower height segments is more practical than changing to the Short Grade–Separated section.

# TALL GRADE-SEPARATED SECTIONS DIMENSION TABLE Max. Height, Y 1'-0" 1'-6" 2'-0" 2'-6" 3'-0" 3'-6" 4'-0" Footing Width, W 3'-3" 3'-6" 3'-9" 4'-0" 4'-3" 4'-6" 4'-6"

#### MEDIAN BARRIER - GRADE-SEPARATED

LAST REVISION 11/01/18

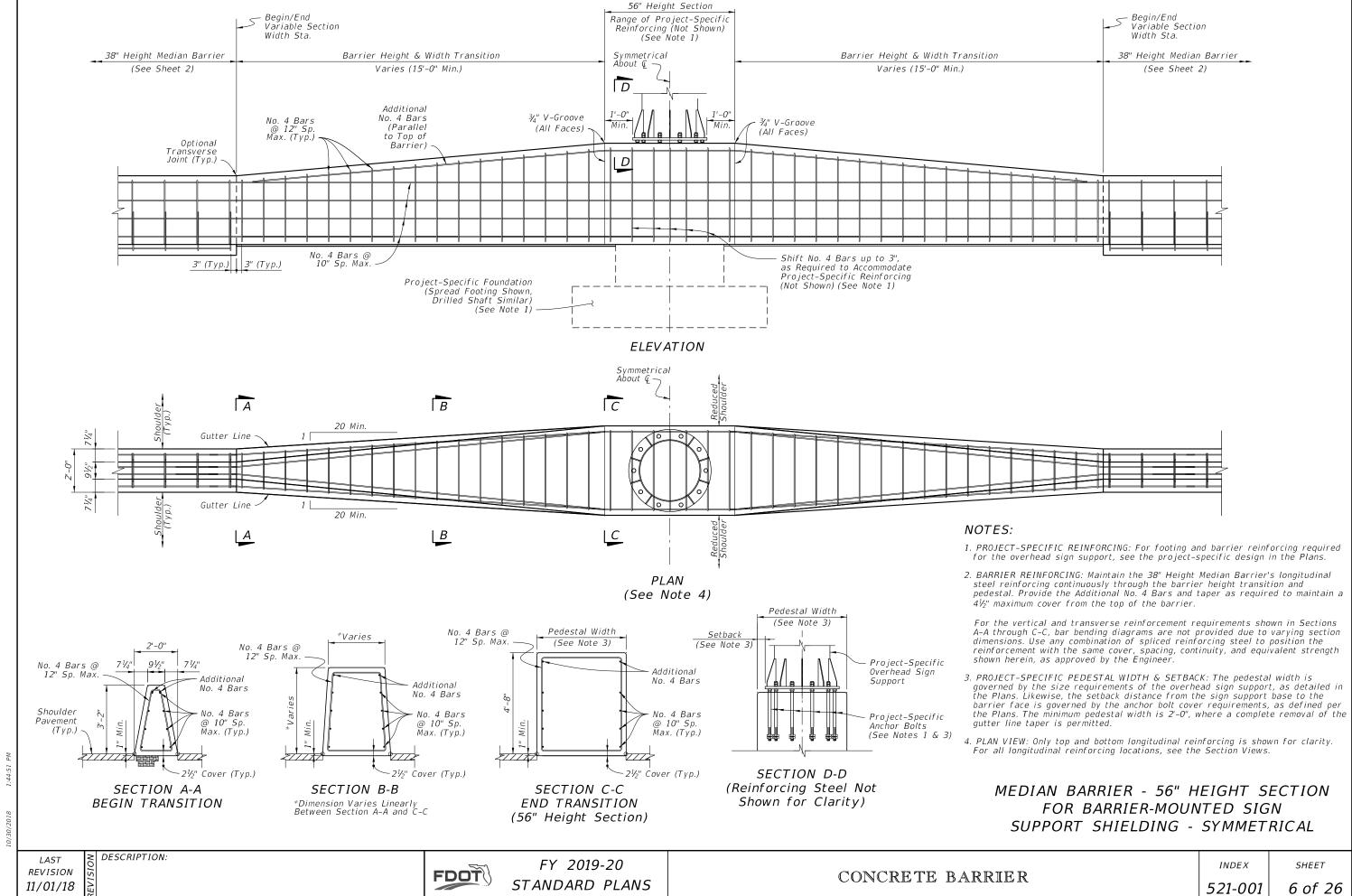
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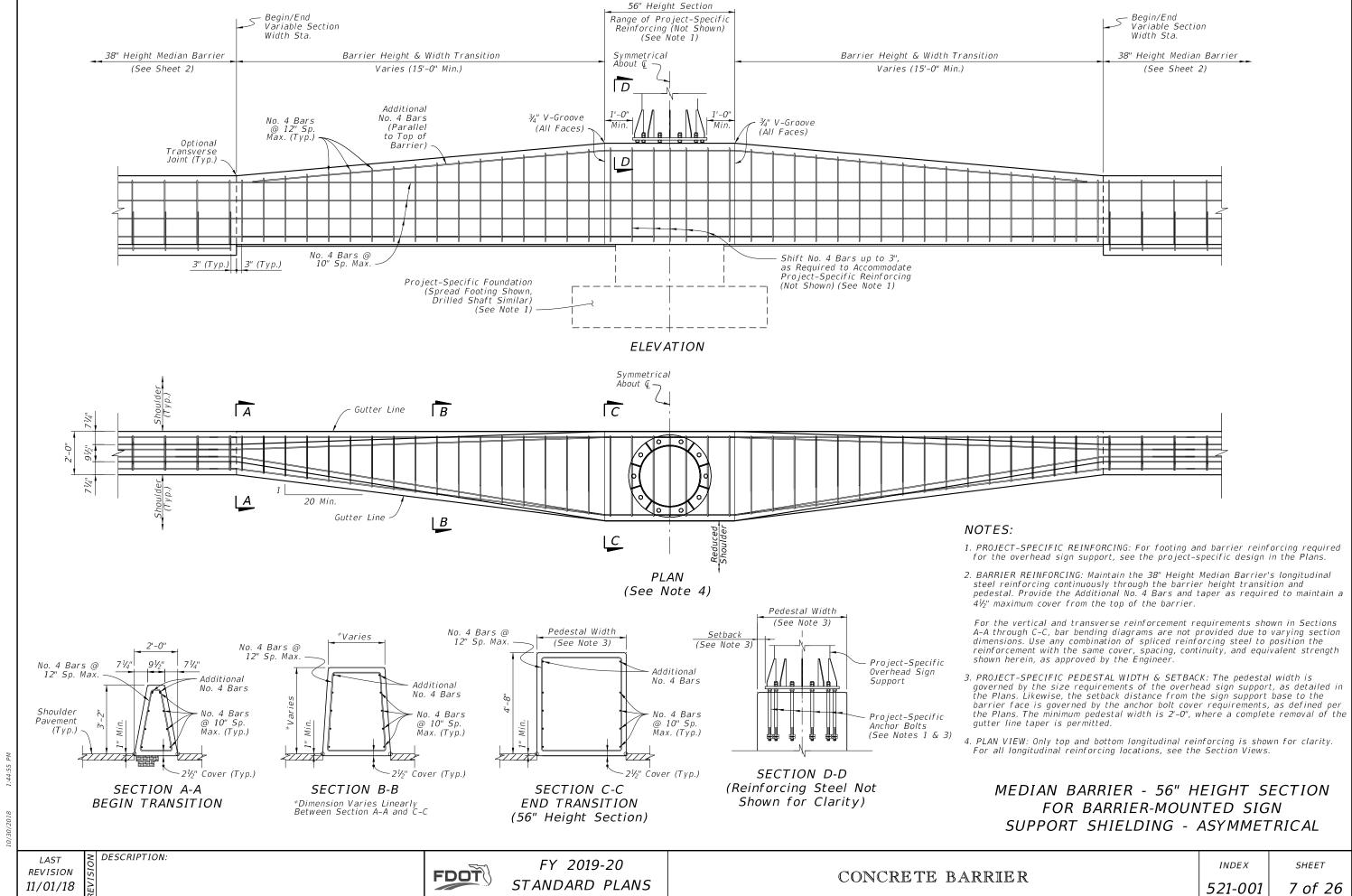
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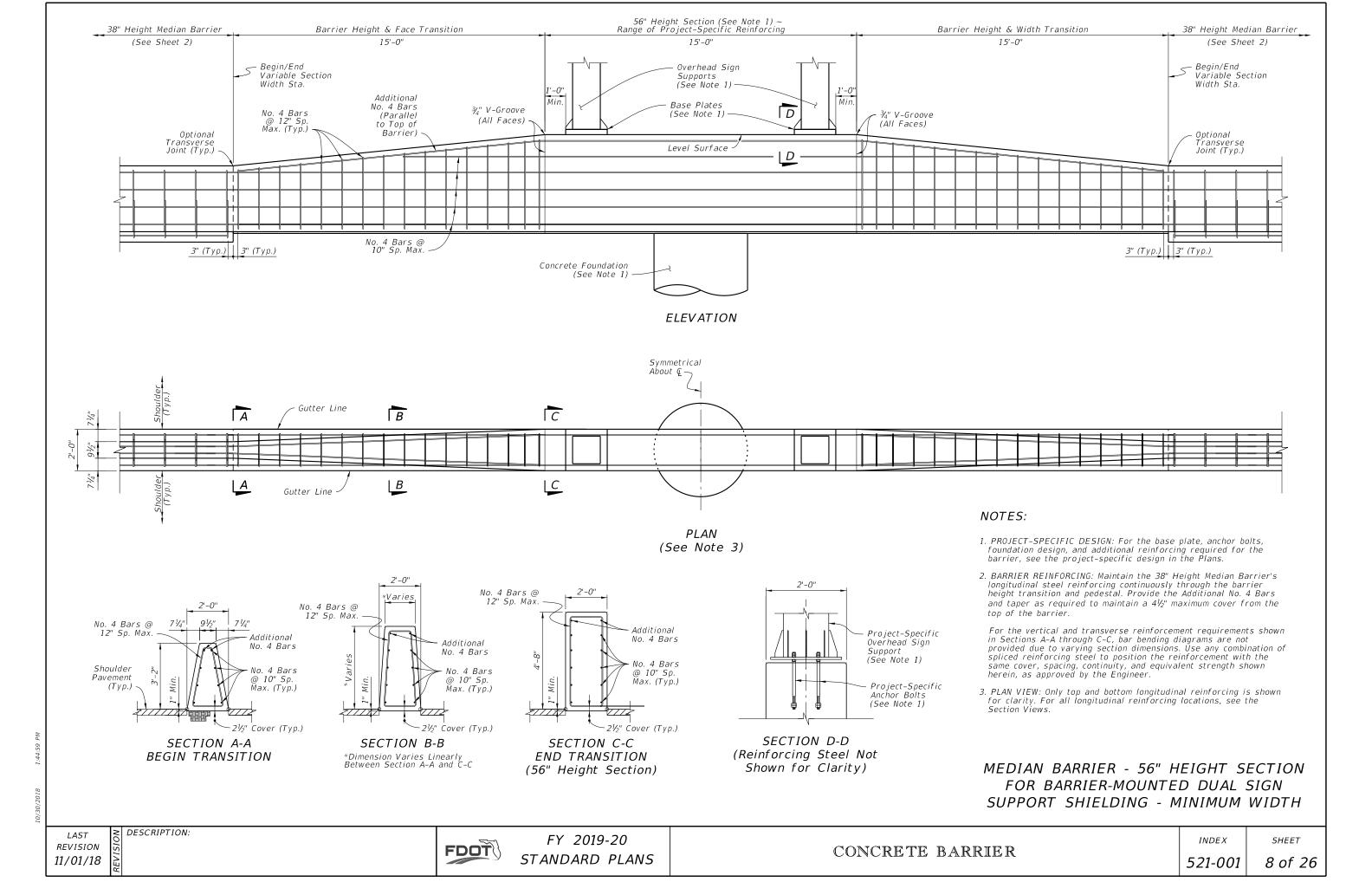
FY 2019-20 STANDARD PLANS CONCRETE BARRIER

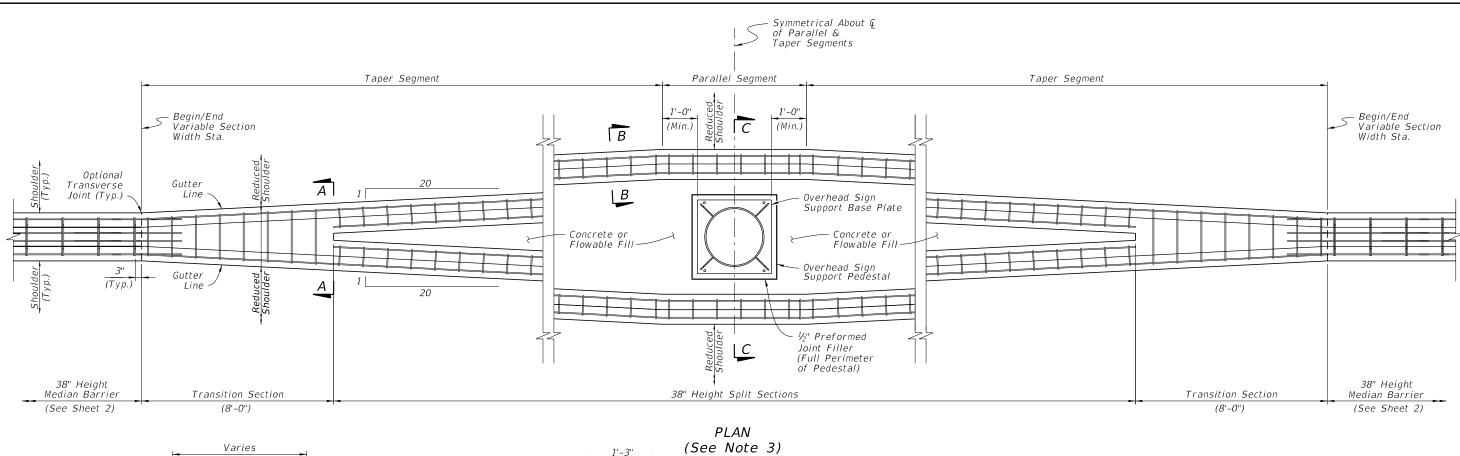
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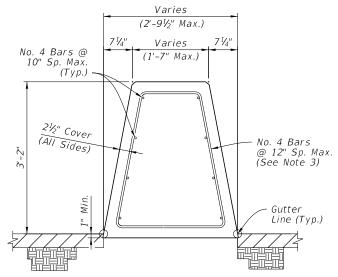
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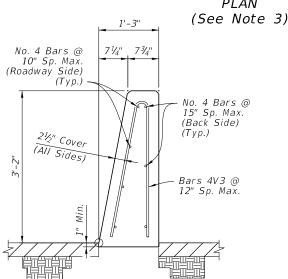




SECTION A-A TRANSITION SECTION (AT BEGIN SPLIT SECTIONS)

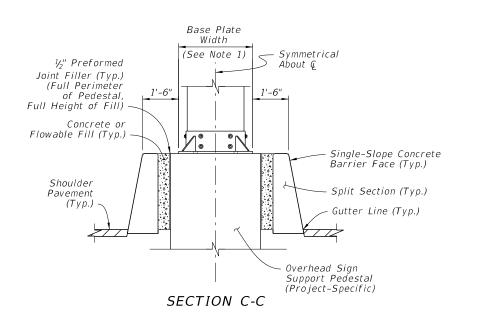
#### NOTES:

- 1. OVERHEAD SIGN SUPPORT: The overhead sign support shown is an example only; see the Plans for the project-specific dimensions and requirements. The overall length and width of the barrier's taper and parallel segments is governed by the overhead sign support dimensions as defined in the Plans.
- 2. MULTIPLE SIGN SUPPORTS: The parallel segment may be lengthened to accommodate multiple sign supports, with the approach and trailing tapers located 1 foot, measured longitudinally, upstream and downstream from the first and last sign support bases, respectively.
- 3. PLAN VIEW: Only outermost longitudinal reinforcing is shown for clarity. For all longitudinal reinforcing locations, see the Section Views.



SECTION B-B 38" HEIGHT SPLIT SECTION (OPPOSITE SIDE SIMILAR BY OPPOSITE HAND)

- 4. STIRRUP BARS: For the vertical and transverse reinforcement requirements shown in Sections A-A, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.
- 5. CONCRETE OR FLOWABLE FILL: Use Class NS Concrete in accordance with Specification 347 or Non-Excavatable Flowable Fill in accordance with Specification



MEDIAN BARRIER - 38" HEIGHT SPLIT SECTION FOR STAND-ALONE SIGN SUPPORT SHIELDING

REVISION 11/01/18

DESCRIPTION:

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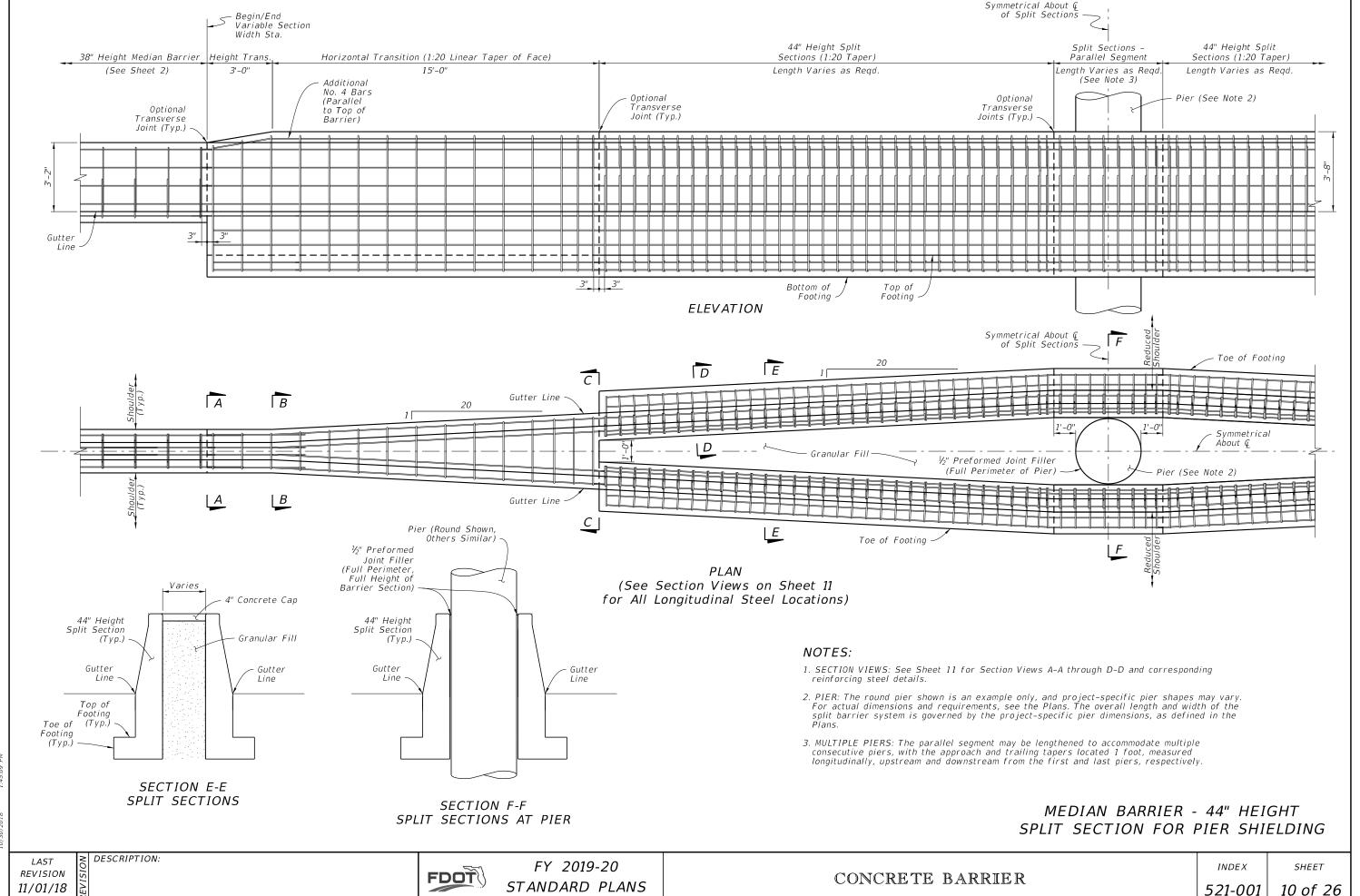
FY 2019-20 STANDARD PLANS

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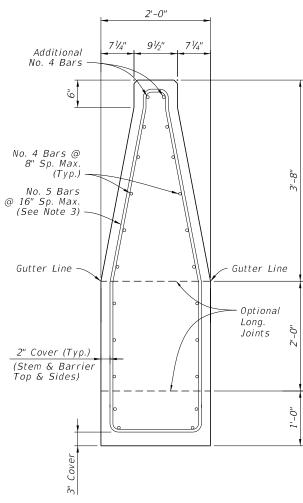
SHEET

CONCRETE BARRIER

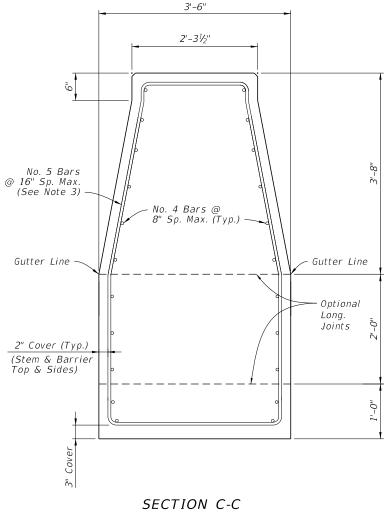
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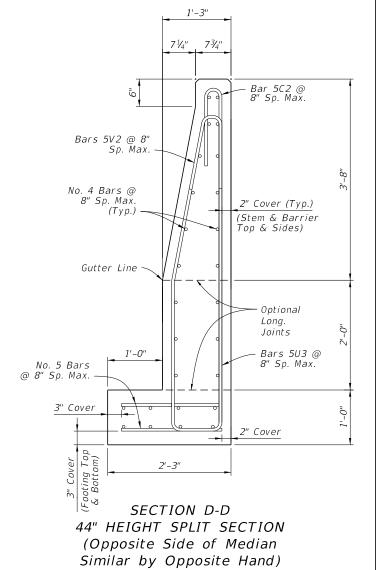
BEGIN HEIGHT TRANSITION (show spliced bars)



SECTION B-B END HEIGHT TRANSITION BEGIN WIDTH TRANSITION



**END WIDTH TRANSITION** BEGIN SPLIT SECTIONS



Concrete Qty. = 0.30 CY/FT Steel Qty. = 52.6 LB/FT

# NOTES:

DESCRIPTION:

- 1. GENERAL: Work with the Plan and Elevation views on Sheet 10.
- 2. LONGITUDINAL REINFORCING CONTINUITY: Maintain all longitudinal steel reinforcing shown in Section C-C continuously into Section D-D (spliced where required). The additional longitudinal reinforcing shown in Section D-D does not require continuity into Section C-C, and it starts 3" from the construction joint or edge of concrete per the details on Sheet 10.
- 3. STIRRUP BARS: For the vertical and transverse reinforcement requirement shown, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

MEDIAN BARRIER - 44" HEIGHT SPLIT SECTION FOR PIER SHIELDING - DETAILS

**REVISION** 11/01/18

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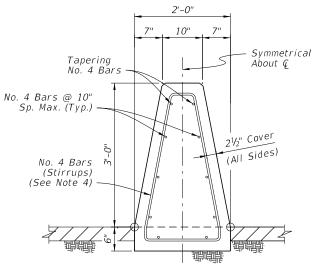
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CONCRETE BARRIER

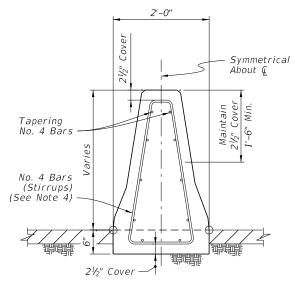
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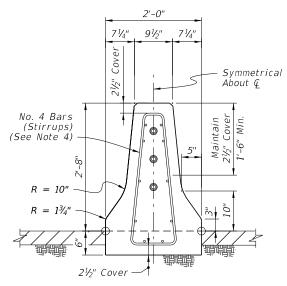
SECTION A-A BEGIN TRANSITION - OPTION 'A' MATCH SINGLE-SLOPE 38" HEIGHT MEDIAN BARRIER



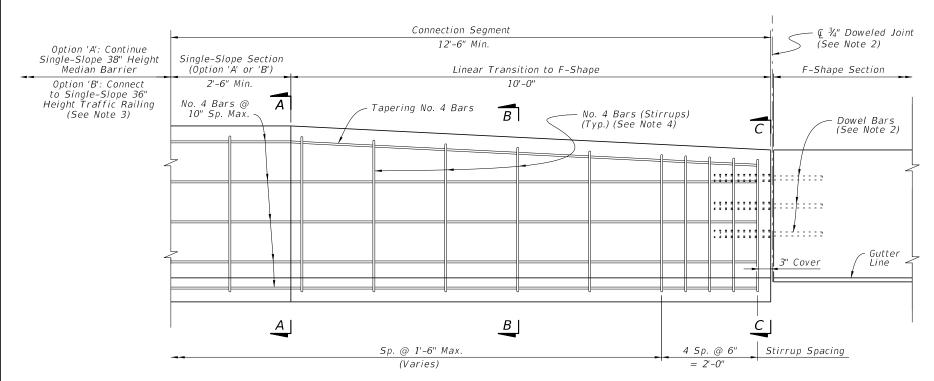
SECTION A-A BEGIN TRANSITION - OPTION 'B' MATCH SINGLE-SLOPE 36" HEIGHT TRAFFIC RAILING (Bridge Applications)



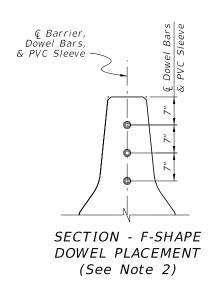
SECTION B-B INTERMEDIATE SECTION OF LINEAR TRANSITION



SECTION C-C **END TRANSITION** MATCH 32" HEIGHT F-SHAPE SECTION



ELEVATION (Reverse Direction Similar by Opposite Hand)



#### NOTES:

- 1. GENERAL: Construct the Connection Segment as required per the Plans to connect existing F-Shape sections to Single-Slope Median Barrier or Traffic Railing sections. Construct Option 'A' or 'B' as required to match the heights of the connecting sections.
- 2. DOWELED JOINT: Install Dowel Bars per the Dowel Details on Sheet 2.
- 3. TRAFFIC RAILING CONNECTION: For the Option 'B' connection, use a Doweled Joint per Sheet 2 and the additional Free End Reinforcing with reduced bar spacing per Sheet 3.
- 4. STIRRUP BARS: For the vertical and transverse reinforcement requirements shown, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

MEDIAN BARRIER - CONNECTION TO F-SHAPE

**REVISION** 11/01/18

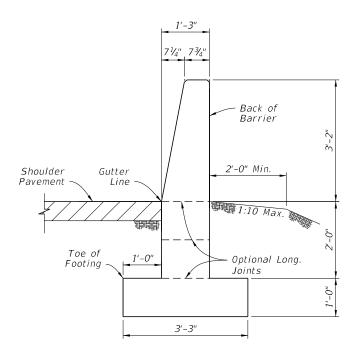
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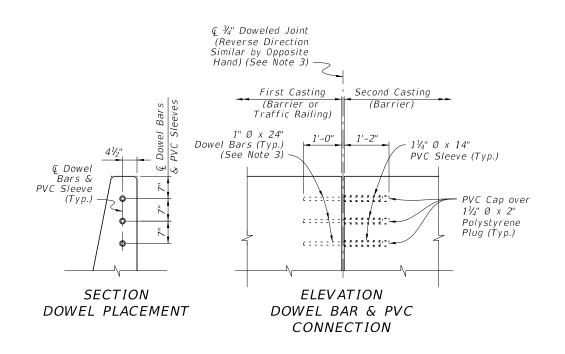
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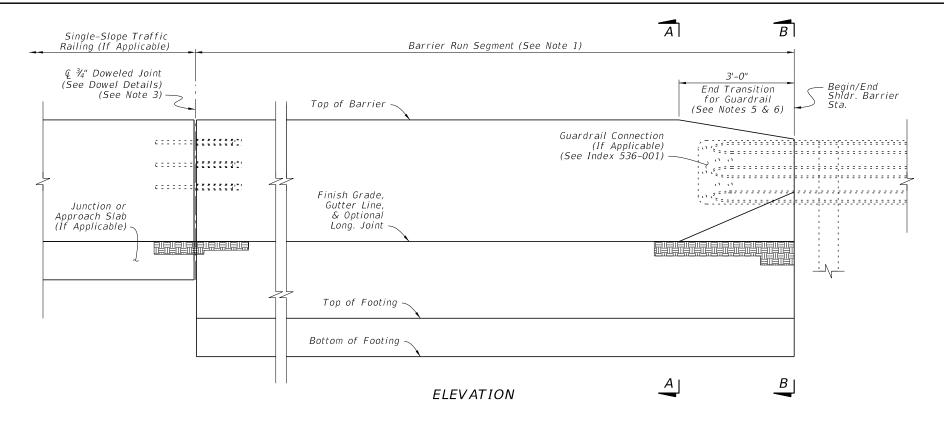
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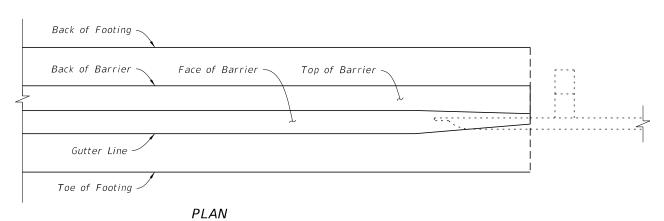


SECTION A-A 38" HEIGHT SHOULDER BARRIER (See Sheet 14 for Reinforcing Steel Details)



DOWEL DETAILS





# NOTES:

- 1. BARRIER RUN SEGMENT: Either the 38" Height Shoulder Barrier or the differing Shoulder Barrier sections shown throughout the Index may be placed within this segment as required per the Plans.
- 2. SECTION VIEWS: For additional Views A-A and B-B, see Sheet 14.
- 3. DOWELED JOINTS: See the General Notes on Sheet 1 for usage of joint types. Place steel reinforcing with a longitudinal 3" cover adjacent to the joint face in the barrier. Use ASTM A36 smooth round bars with hot-dip galvanization.

For the dowel connection into the first casting, the dowel may be cast-in-place for new concrete or placed into a  $1\frac{1}{6}$ "  $\times$  x 13" ( $\pm$   $\frac{1}{2}$ ") drilled hole for cured concrete. For drilled holes larger than 11/8"O, secure the dowel with adhesive in accordance with Specification 416. No load testing is required.

For the dowel connection into the second casting, use a 1½" NPS Schedule 80 PVC pipe with a sealed cap, cast-in-place as shown.

- 4. TRAFFIC RAILING CONNECTIONS: Align the barrier and Traffic Railing faces and connect with the ¾" Doweled Joint.
- 5. GUARDRAIL CONNECTIONS: Connect Guardrail using the Transition Connections to Rigid Barrier per Index 536-001.
- 6. CRASH CUSHION CONNECTIONS: Connect Crash Cushions per Index 544-001 in conjunction with the 3'-0" End Transition for Guardrail as shown herein.
- 7. FREE ENDS: When the barrier end does not terminate with a Traffic Railing Connection, Guardrail Connection, or Crash Cushion Connection as called for in the Plans, terminate in accordance with the Free End Reinforcing Note on Sheet 14.

# SHOULDER BARRIER

LAST **REVISION** 11/01/18

DESCRIPTION:

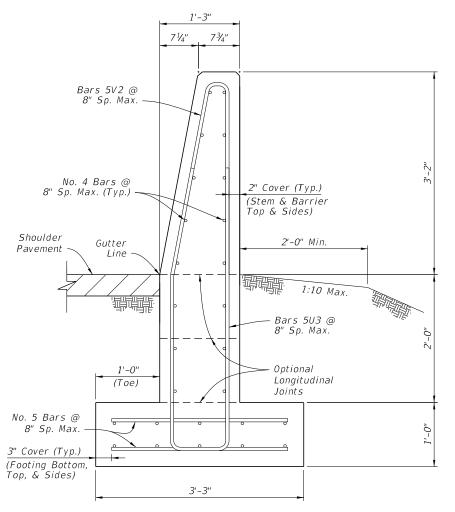
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FY 2019-20 STANDARD PLANS

CONCRETE BARRIER

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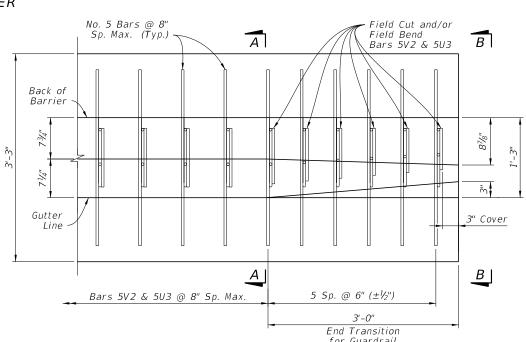


SECTION A-A 38" HEIGHT SHOULDER BARRIER

> Concrete Qty. = 0.32 CY/FT Steel Qty. = 50.9 LB/FT

#### NOTES:

- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 13. The Section Option footings shown on Sheet 15 may be substituted where called for in the Plans.
- 2. FREE END REINFORCING: Where shown in the Plans, terminate the 38" Height Barrier section with a transverse vertical end face. Reduce the spacing of Bars 5V2 and 5U3 to 6" for 5 Spaces, placed with 3" cover from the barrier's end face.
- 3. BAR BENDING DIAGRAMS: For additional details for bars 5V2 and 5U3, see the Bar Bending Diagrams on Sheet 26.



VIEW B-B REDUCED SECTION OF **END TRANSITION** FOR GUARDRAIL (End of Barrier)

3'-3"

6½"

Bars 5V2 @ 6" Sp. (Field Bend Top & Cut Bottom to Lap

Shoulder

Pavement

No. 5 Bars @ 6" Sp.

3" Cover (Typ.)

(Footing Bottom,

Top, & Sides)

with Bars 5U3)

Cover Varies | |

21/2" Cover

2" Cover (Min.)

1'-0"

(Toe)

87/8"

No. 4 Bars Tapered Down with Barrier Height

No. 4 Bars

2'-0" Min.

Optional Property

Joints.

Longitudinal

@ 8" Sp. Max. (Typ.)

1:10 Max.

Bars 5U3 @ 6" Sp. (Field Bend Bottom

to Align with Bars 5V2)

2" Cover

PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (Longitudinal Steel Not Shown for Clarity)

SHOULDER BARRIER - REINFORCING DETAILS

LAST **REVISION** 11/01/18

DESCRIPTION:

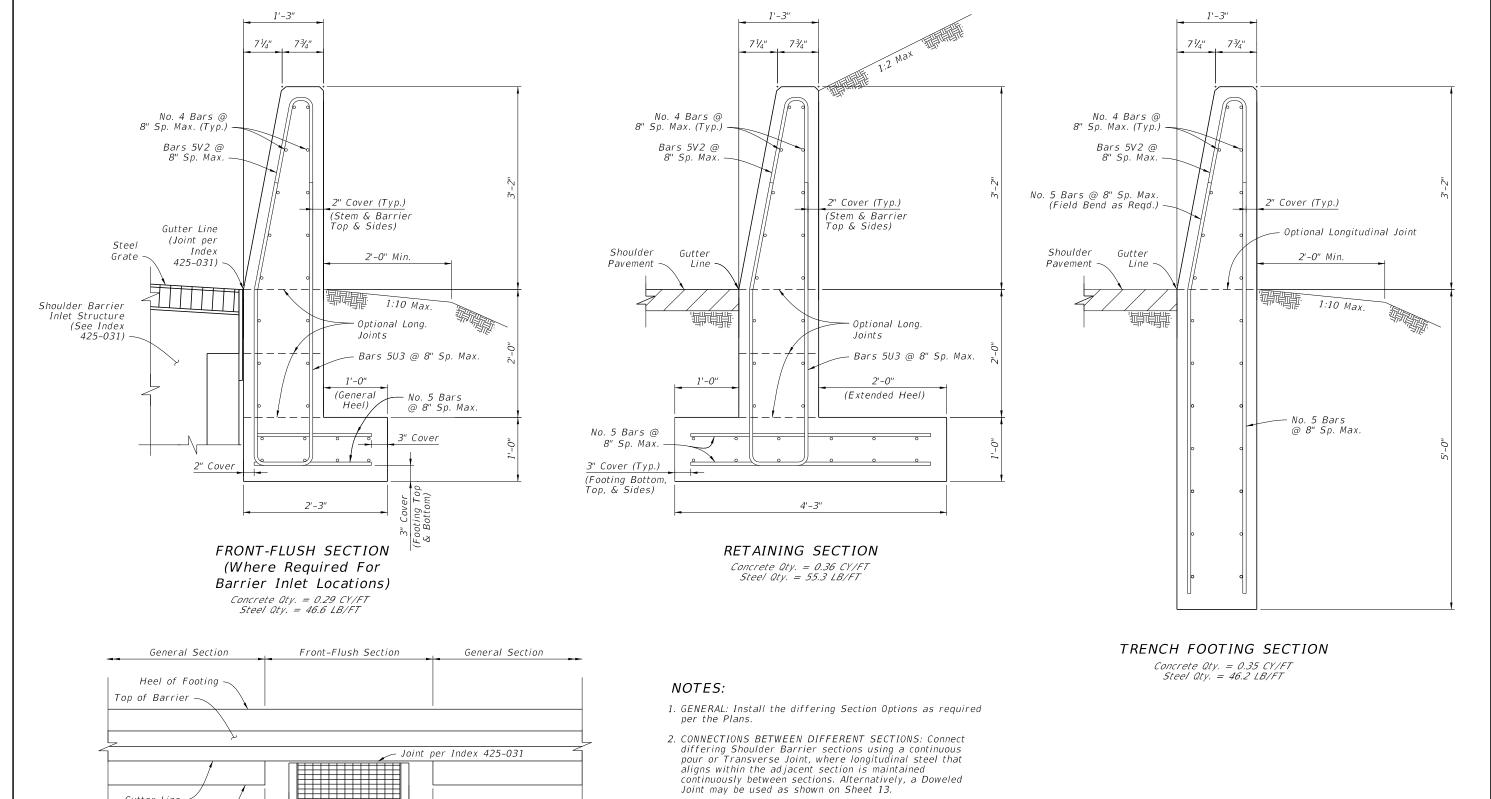
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FY 2019-20 STANDARD PLANS

CONCRETE BARRIER

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Gutter Line Toe of Footing -Shoulder Barrier

> FRONT-FLUSH SECTION - PLAN VIEW (Not Applicable for Trench Footing Sections)

3. FLUSH RETAINING SECTION COMBINATION: Where Barrier Inlets are required in retaining segments, install the Flush Section, except replace the 1'-0" General Heel with the 2'-0" Extended Heel as shown in the Retaining Section. Use longer lateral reinforcing bars of 2'-10" length to maintain the cover shown.

SHOULDER BARRIER - SECTION OPTIONS

**REVISION** 11/01/18

DESCRIPTION:

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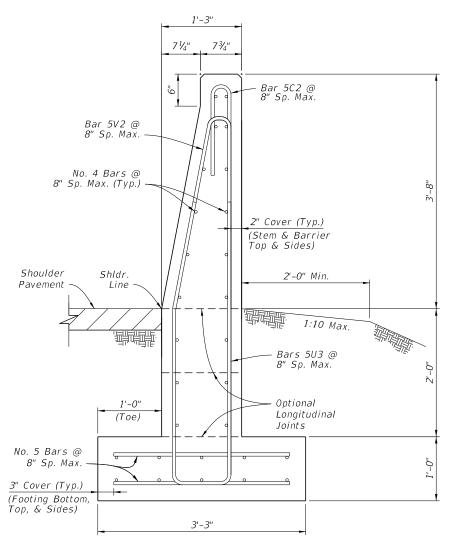
Inlet Structure

per Index 425-031

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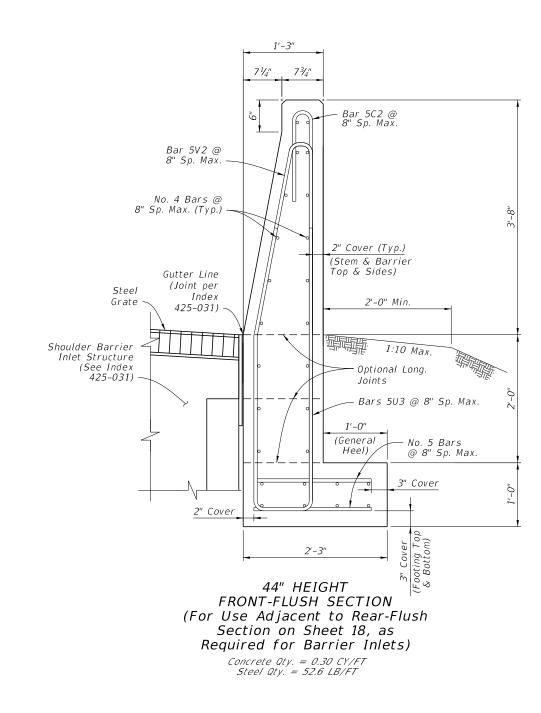


44" HEIGHT SECTION (For Use Adjacent to Rear-Flush Section on Sheet 18)

> Concrete Qty. = 0.34 CY/FT Steel Qty. = 56.8 LB/FT

NOTE:

1. GENERAL: See the applicable Notes on Sheet 15.



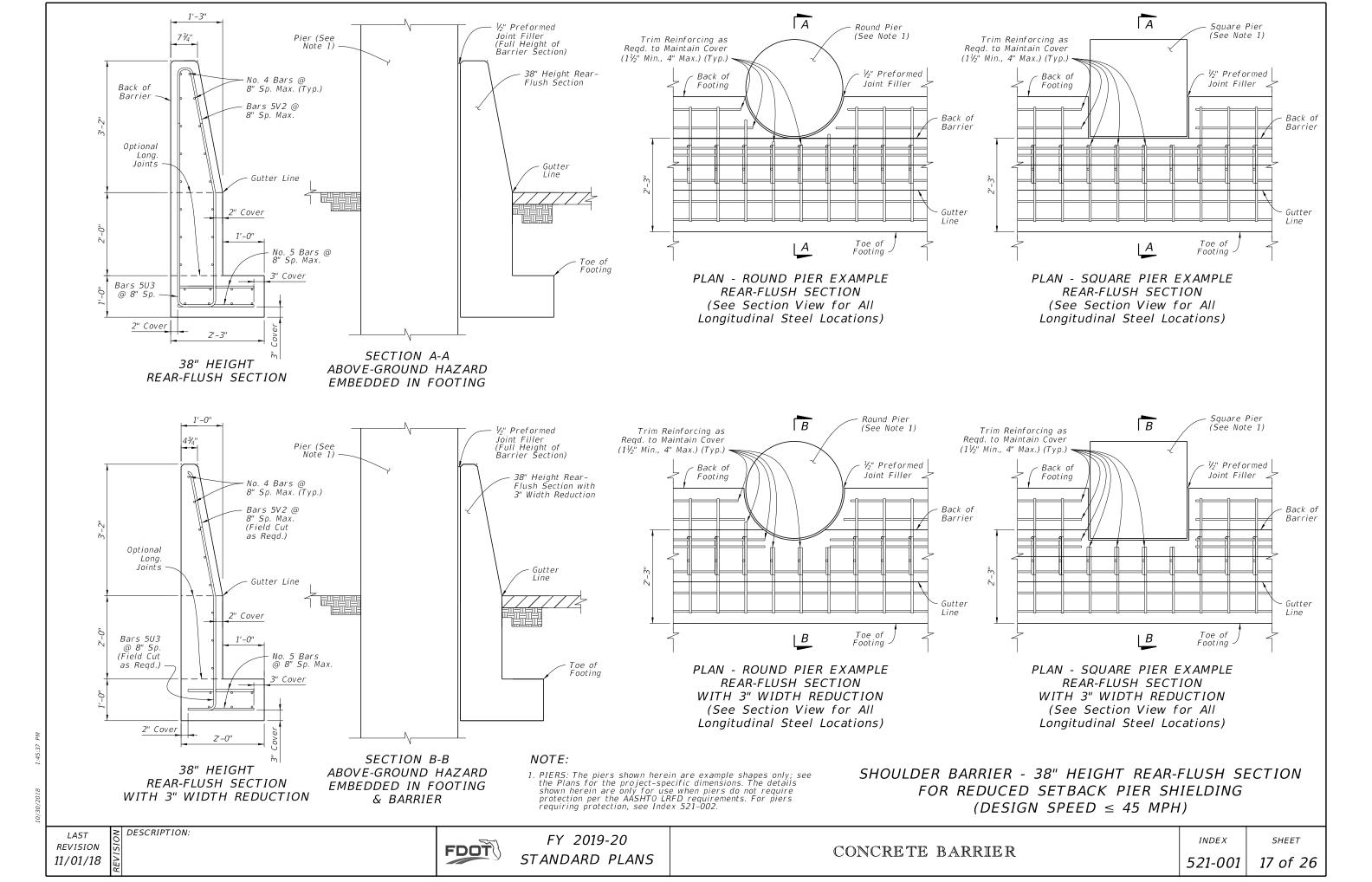
SHOULDER BARRIER - SECTION OPTIONS (CONTINUED)

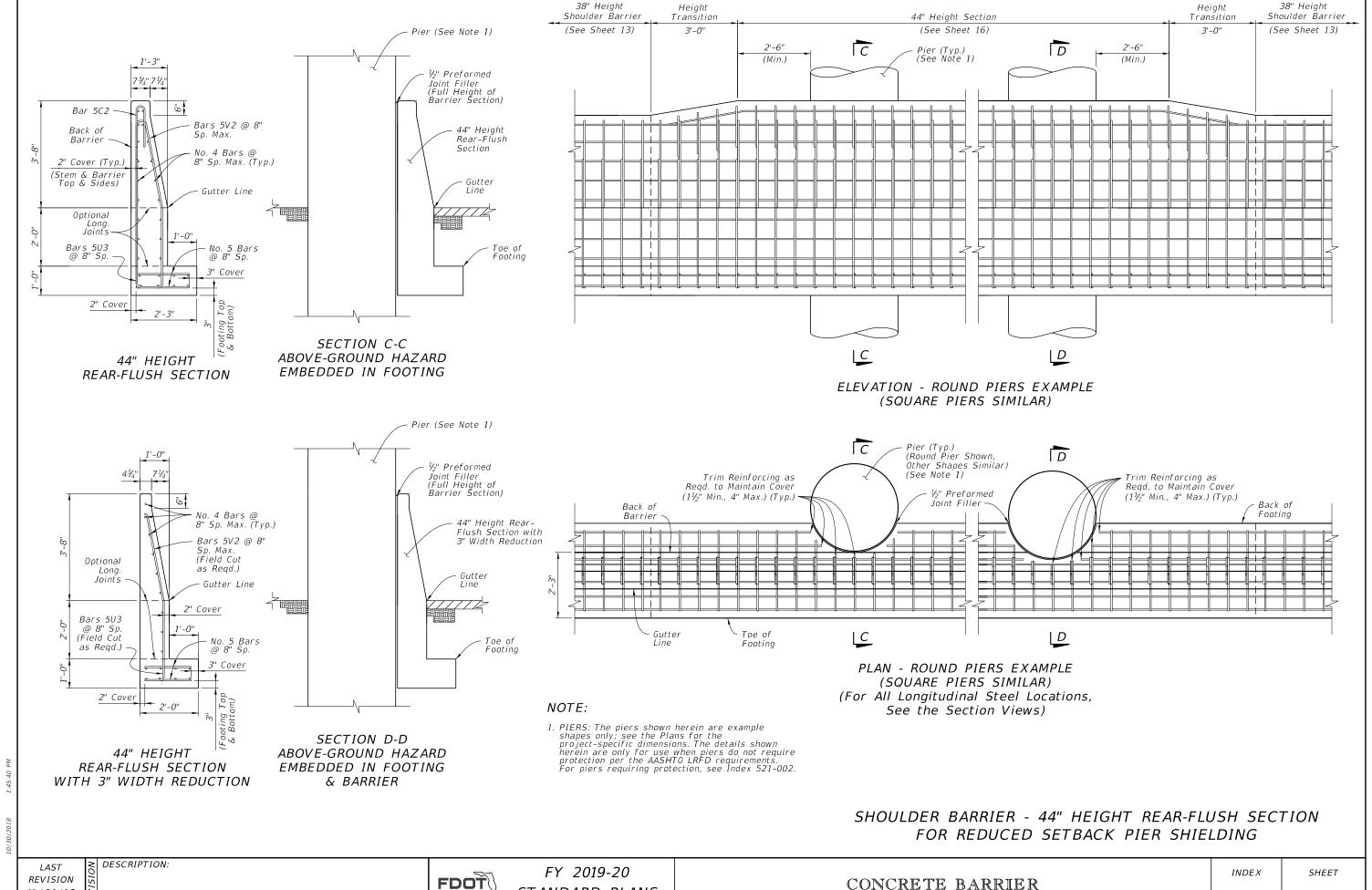
LAST REVISION 11/01/18

DESCRIPTION:

FDOT

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STANDARD PLANS

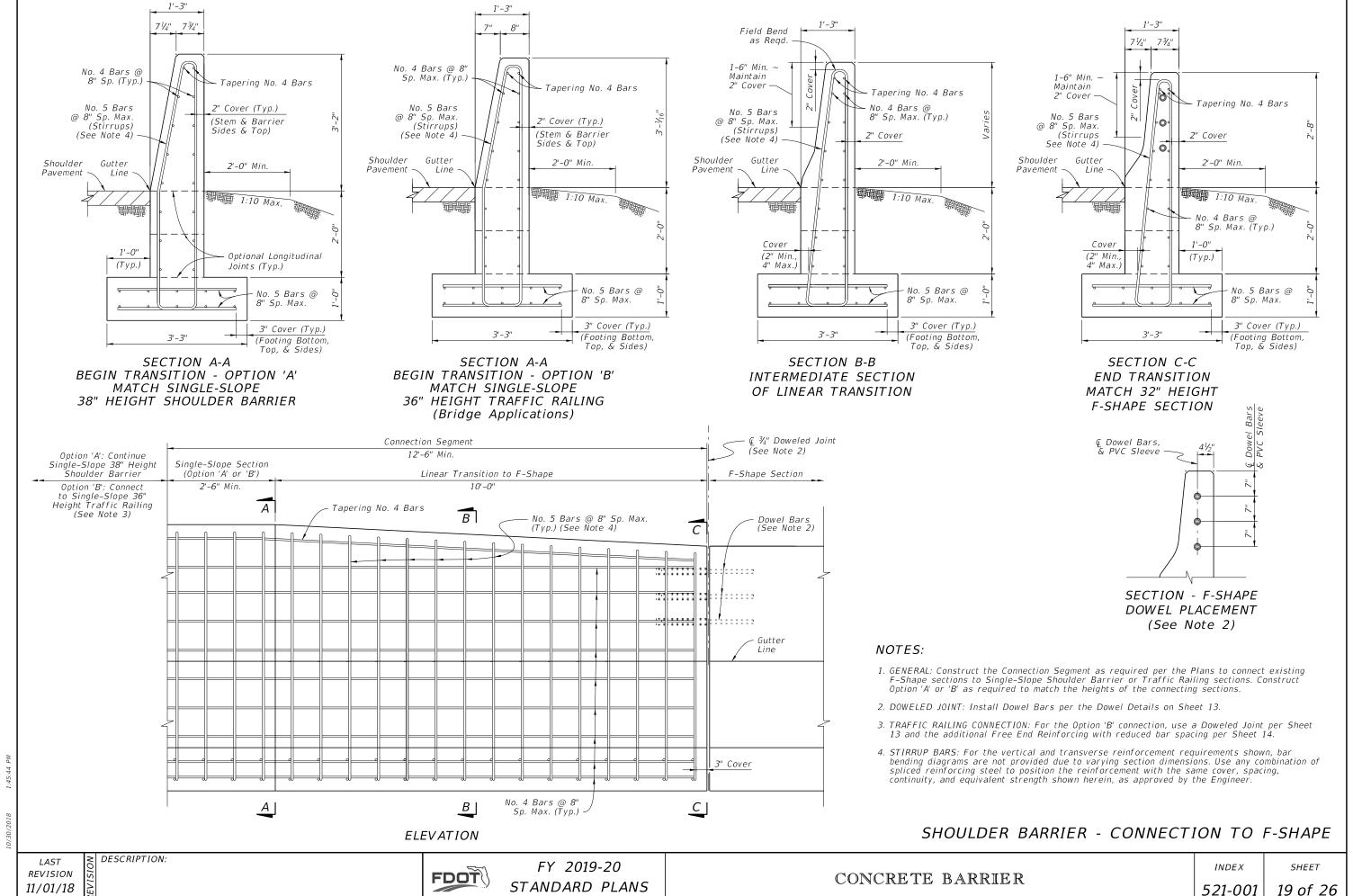
38" Height

38" Height

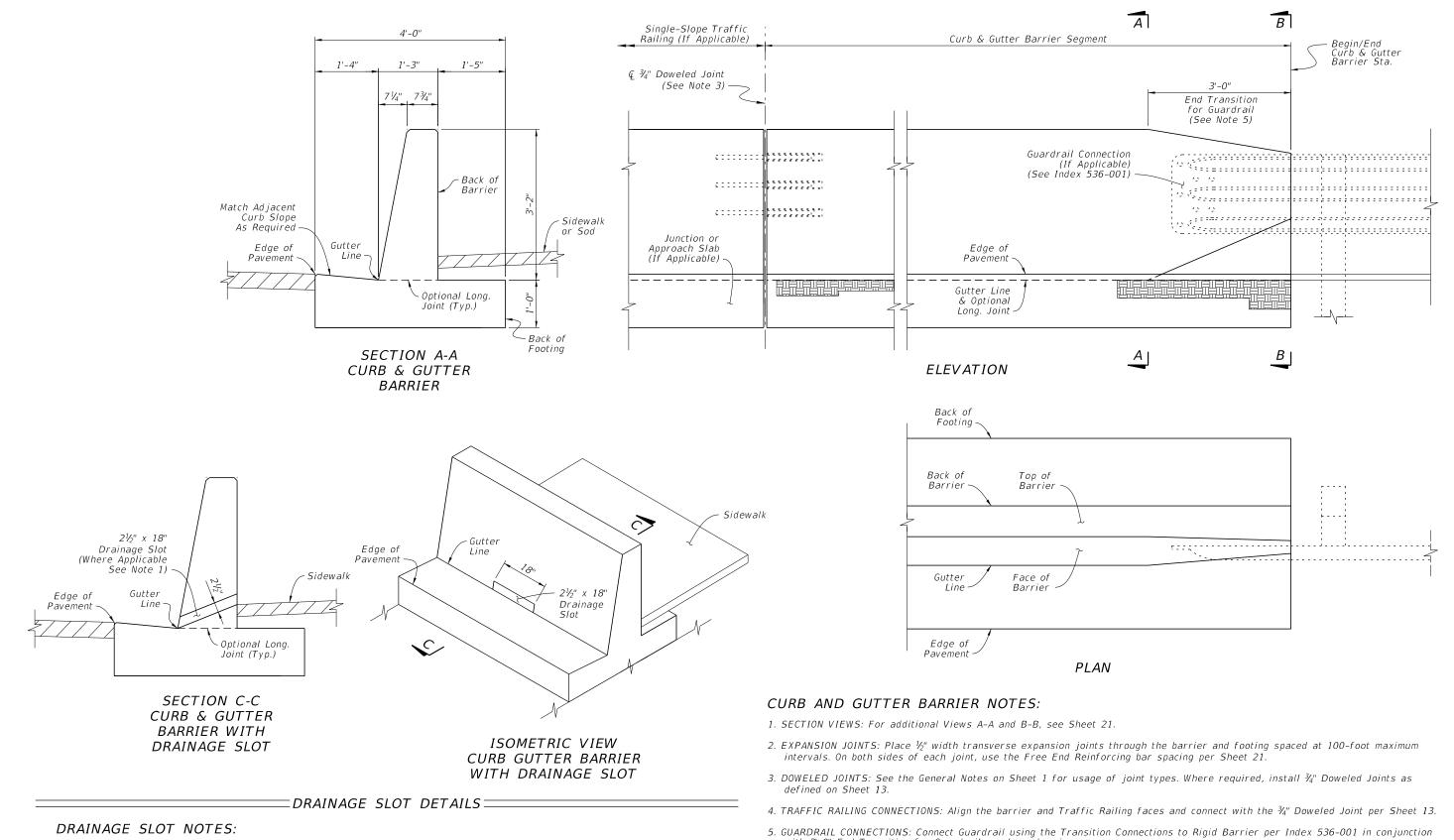
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- 1. GENERAL: Place  $2\frac{1}{2}$ " x 18" Drainage Slots at locations and/or spacing called for in the Plans.
- 2. STEEL REINFORCEMENT CONFLICT: When the Drainage Slot encounters a conflict with reinforcing steel, shift or cut the reinforcing steel to provide  $2\frac{1}{2}$ "(±  $\frac{1}{2}$ ") of concrete cover for the reinforcing around the Drainage Slot. If cutting the vertical bars, maintain 8" bar spacing. If shifting the vertical bars, move the bars from the standard 8" spacing location to the closest end of the drainage slot (distributing additional vertical reinforcement evenly on each side of the Drainage Slot).
- with 3'-0" End Transition for Guardrail as shown herein.
- 6. FREE ENDS: When the barrier end does not terminate with a Traffic Railing connection or Guardrail connection as called for in the Plans, terminate the barrier in accordance with the Free End Reinforcing Note on Sheet 21.

# CURB AND GUTTER BARRIER

REVISION 11/01/18

DESCRIPTION:

**FDOT** 

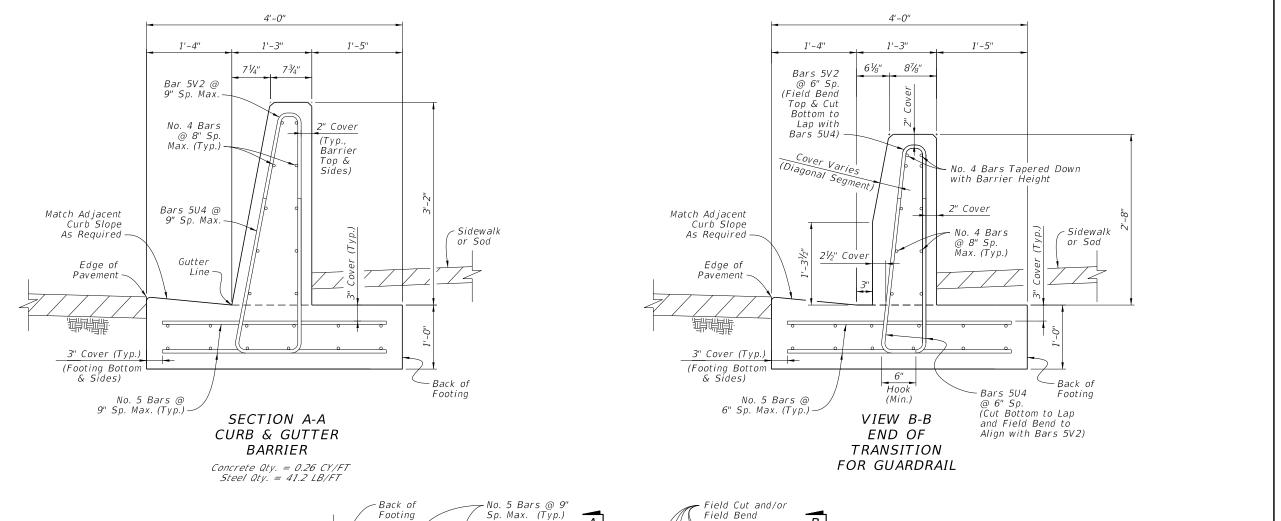
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CONCRETE BARRIER

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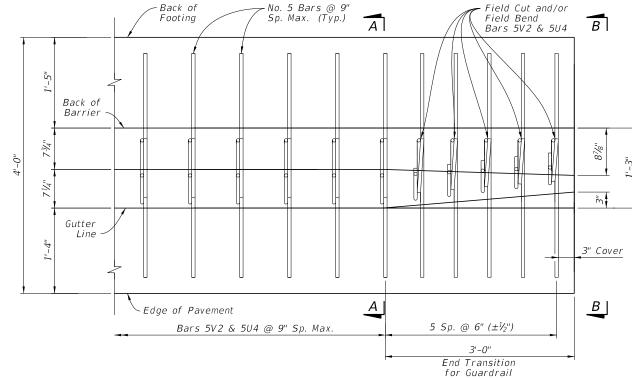
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- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 20.
- 2. FREE END REINFORCING: Where shown in the Plans, terminate the 38" Curb & Gutter Barrier section with a transverse vertical end face. Reduce the spacing of Bars 5V2 and 5U4 to 6" for 5 Spaces, placed with 3" cover from the barrier's
- 3. BAR BENDING DIAGRAMS: For additional details for bars 5V2 and 5U4, see the Bar Bending Diagrams on Sheet 26.



PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (Longitudinal Steel Not Shown for Clarity)

CURB AND GUTTER BARRIER -REINFORCING DETAILS

11/01/18

DESCRIPTION:

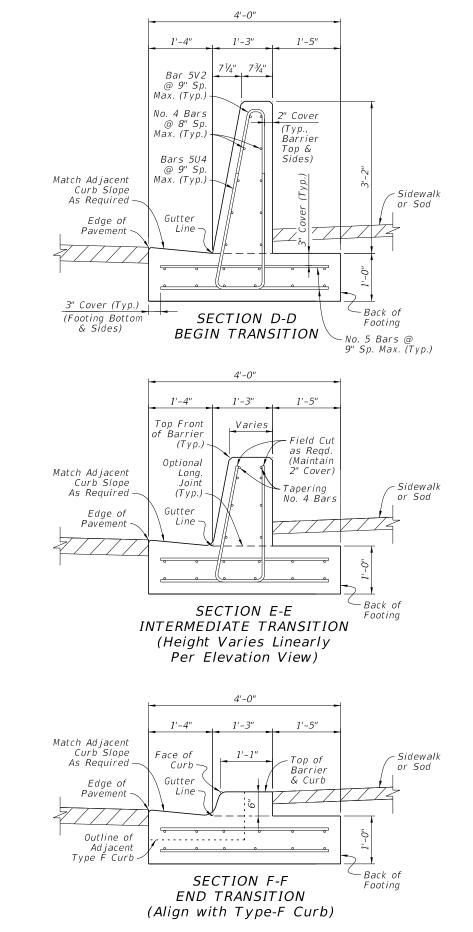
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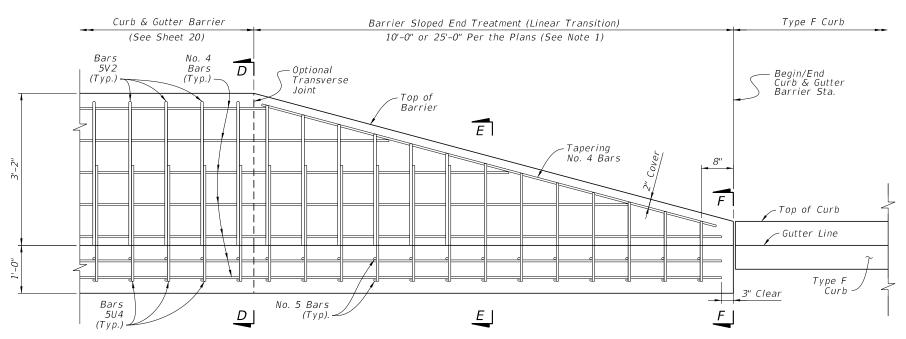
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CONCRETE BARRIER

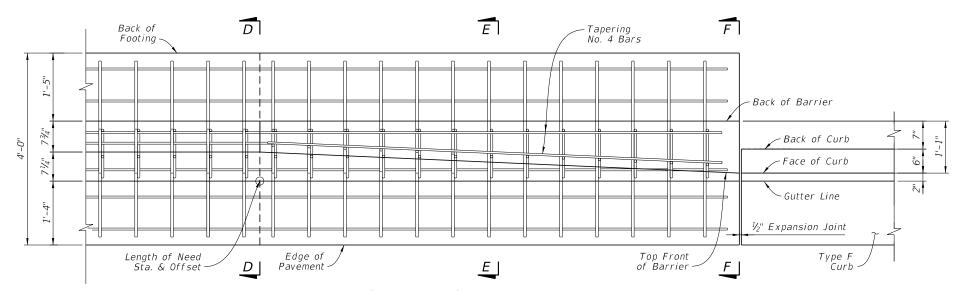
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ELEVATION - CURB AND GUTTER BARRIER SHOWING SLOPED END TREATMENT (Approach and Trailing End Similar by Opposite Hand)



PLAN - CURB AND GUTTER BARRIER SHOWING SLOPED END TREATMENT (Approach and Trailing End Similar by Opposite Hand; See Sections for All Longitudinal Steel Locations)

# NOTES:

- 1. GENERAL: Install a Sloped End Treatment only where called for in the Plans, using either a 10'-0" length or 25'-0" length treatment as specified in the Plans. The 10'-O" length option is shown herein, while the 25'-O" length option requires additional trimmed Bars 5U4 & 5V2 at the same 9" longitudinal spacing.
- 2. BAR BENDING DIAGRAMS: For additional details on Bars 5V2 & 5U4, see the Bar Bending Diagrams on Sheet 26.

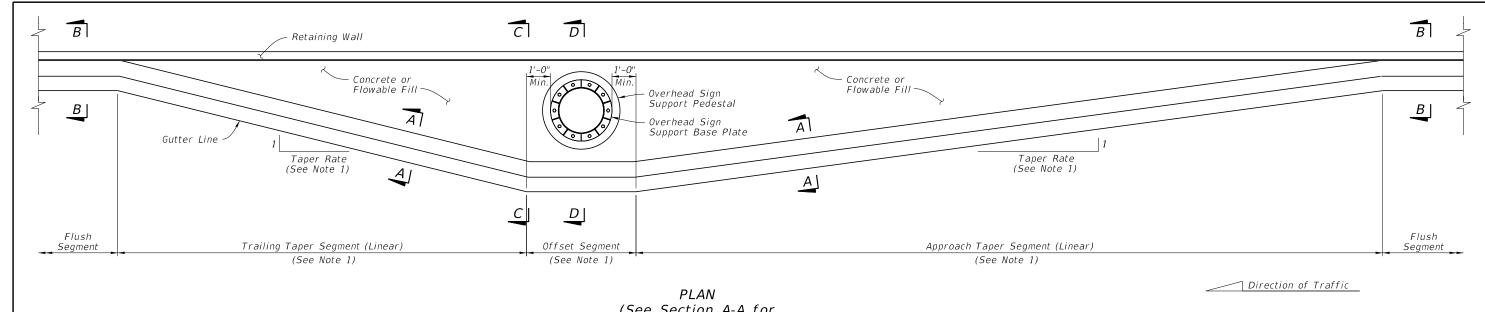
CURB AND GUTTER BARRIER -

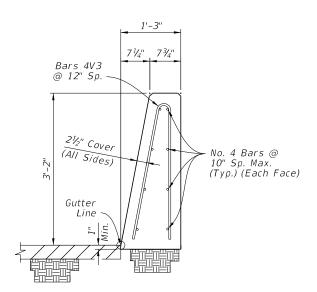
**REVISION** 11/01/18

DESCRIPTION:

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FY 2019-20 STANDARD PLANS SLOPED END TREATMENT

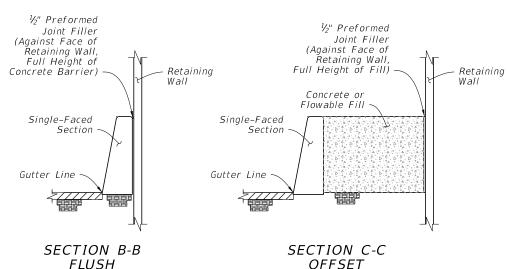


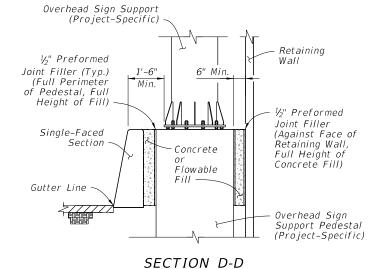


SECTION A-A 38" HEIGHT SINGLE-FACED SECTION (Reverse Side Similar by Opposite Hand)

# (See Section A-A for Barrier Reinforcing)

SEGMENT





OVERHEAD SIGN

**SUPPORT** 

#### NOTES:

DESCRIPTION:

- 1. TAPER SEGMENTS AND OFFSET SEGMENT: The plan view shown is an example only, showing general geometry for the taper segments and offset segment. For the actual segment lengths and corresponding taper rates required, see the barrier placement information in the Plans.
- 2. OVERHEAD SIGN SUPPORT: The overhead sign support shown is an example only; see the Plans for the project-specific dimensions and requirements if applicable.
- 3. CONNECTION TO SHOULDER BARRIER SECTIONS: Connect to Shoulder Barrier sections using a continuous pour or Transverse Joint, where longitudinal steel that aligns within the adjacent section is maintained continuously between sections or has a full lap splice with the adjacent section's longitudinal steel.
- 4. FREE ENDS: Where shown in the Plans, terminate the Single-Faced Section with a transverse end face. Place a stirrup bar with a 3" cover from the end face. Place longitudinal bars with a 3" cover from the end face.

SEGMENT

5. CONCRETE OR FLOWABLE FILL: Use Class NS Concrete in accordance with Specification 347 or Non-Excavatable Flowable Fill in accordance with Specification 121.

> WALL SHIELDING BARRIER -38" HEIGHT SECTION -APPROACH & TRAILING TRANSITION

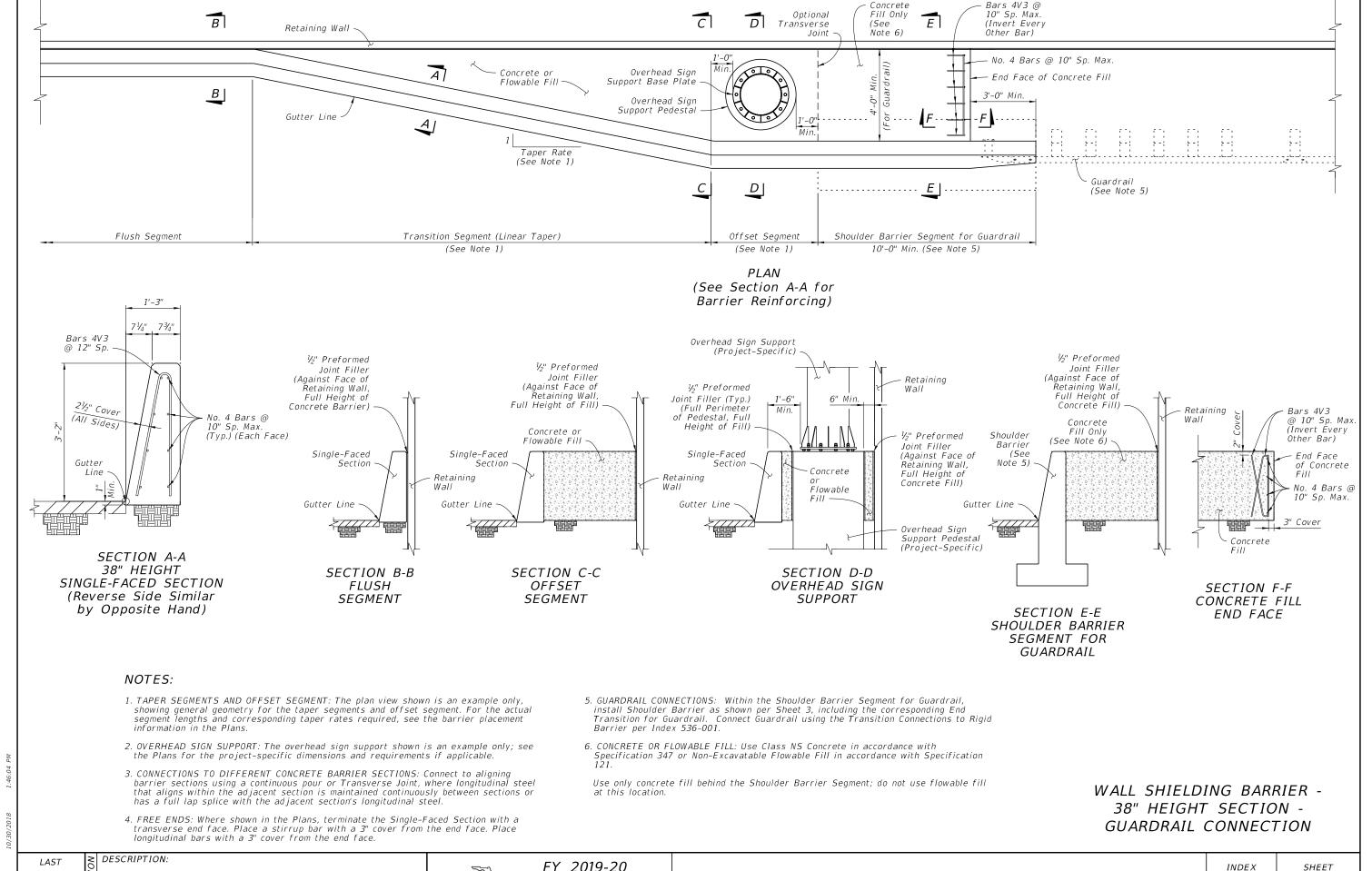
**REVISION** 11/01/18

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**REVISION** 11/01/18

**FDOT** 

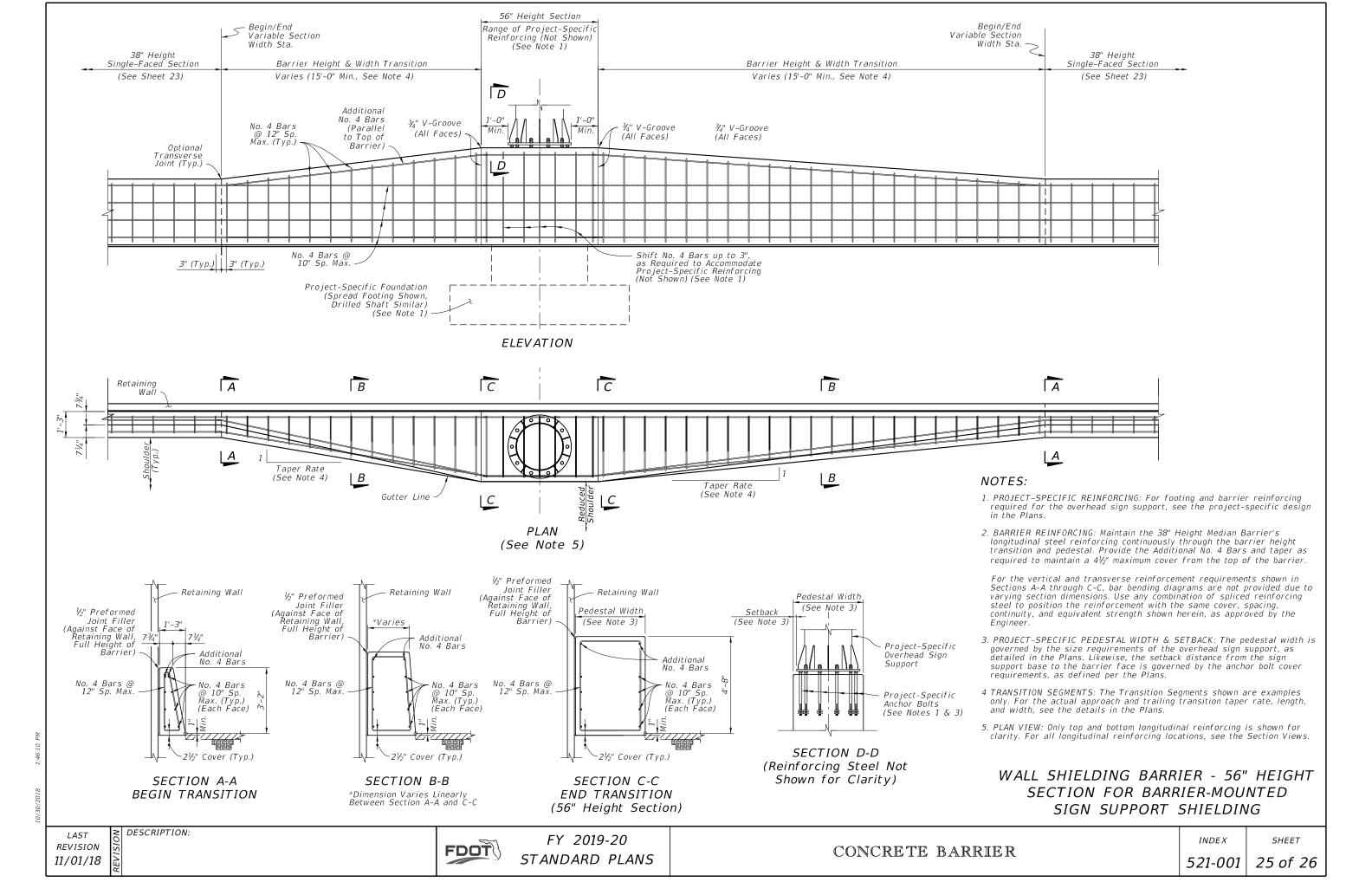
FY 2019-20 STANDARD PLANS

CONCRETE BARRIER

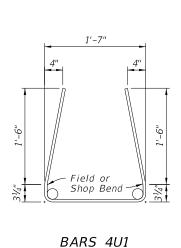
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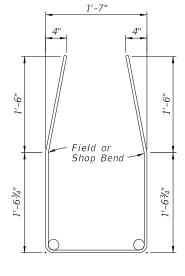
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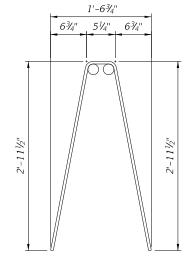
*521-001* |



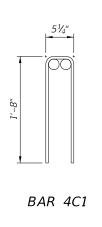
BILL OF REINFORCING STEEL					
MARK	SIZE	LENGTH			
C1	4	3'-8"			
C2	5	3'-0"			
U1	4	5'-1"			
U2	4	7'-8"			
U3	5	9'-7"			
U4	5	5'-9"			
V1	4	6'-4"			
V2	5	6'-3"			
V3	4	5'-10"			
	_				

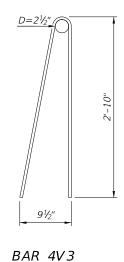






BAR 4V1

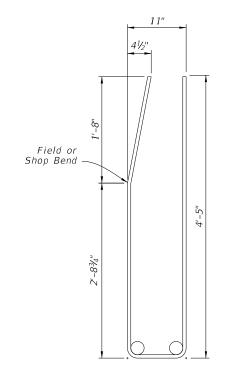




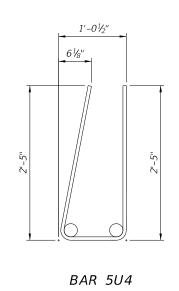
BAR 4U2

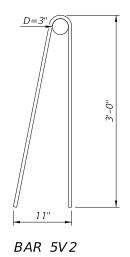
# NOTES:

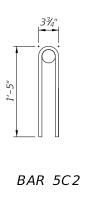
- 1. Work with the Standard Bar Bending Details per Index 415-001.
- 2. All bar dimensions in the bending diagrams are out to out.
- 3. Use standard inner diameters for bar bending unless otherwise shown.











REINFORCING BAR BENDING DIAGRAMS

SHEET NO.	CONTENTS
1	Index Contents; General Notes
2	Example Layouts – Footing Placement and Connections
3	Barrier Plan and Elevation – Connection to Concrete Barrier – Connection to Guardrail
4	Barrier Details - Connection to Concrete Barrier
5	Barrier Details - Connection to Guardrail
6	Barrier Footing Options
7	Crash Wall Details
8	Reinforcing Bar Bending Diagrams

#### GENERAL NOTES:

- 1. CONCRETE: Use Class III or IV concrete unless otherwise called for in the Plans.
- 2. CONSTRUCTION JOINTS: Maintain continuity of reinforcement steel across Construction Joints; reinforcement lap splices are permitted immediately adjacent to joints. Construct all Pier Protection Barrier continuously, with no expansion or contraction joints. Construction Joints are classified herein as Transverse Joints or Longitudinal Joints.

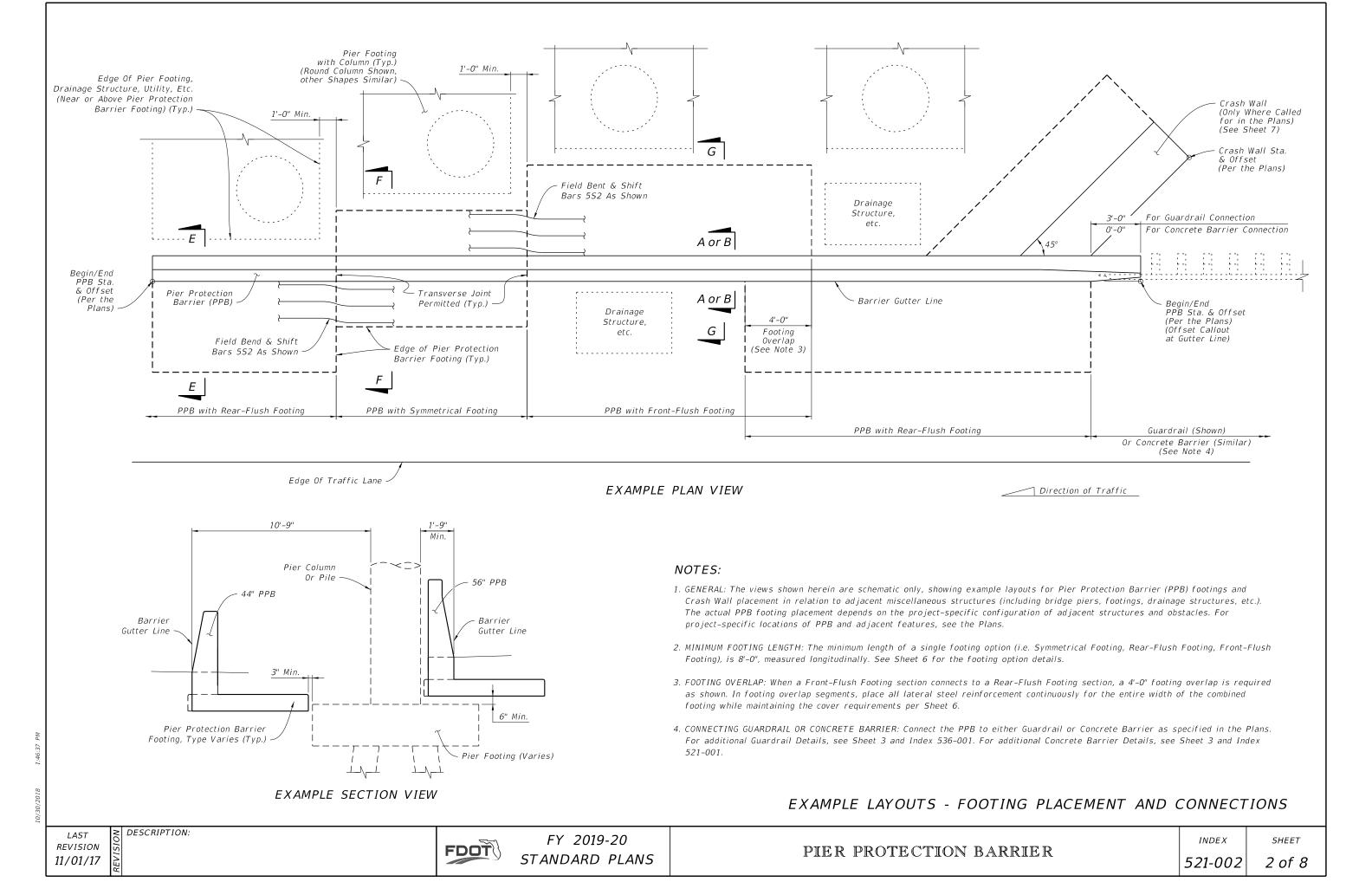
Transverse Joints are permitted at 40 foot or greater intervals along the barrier.

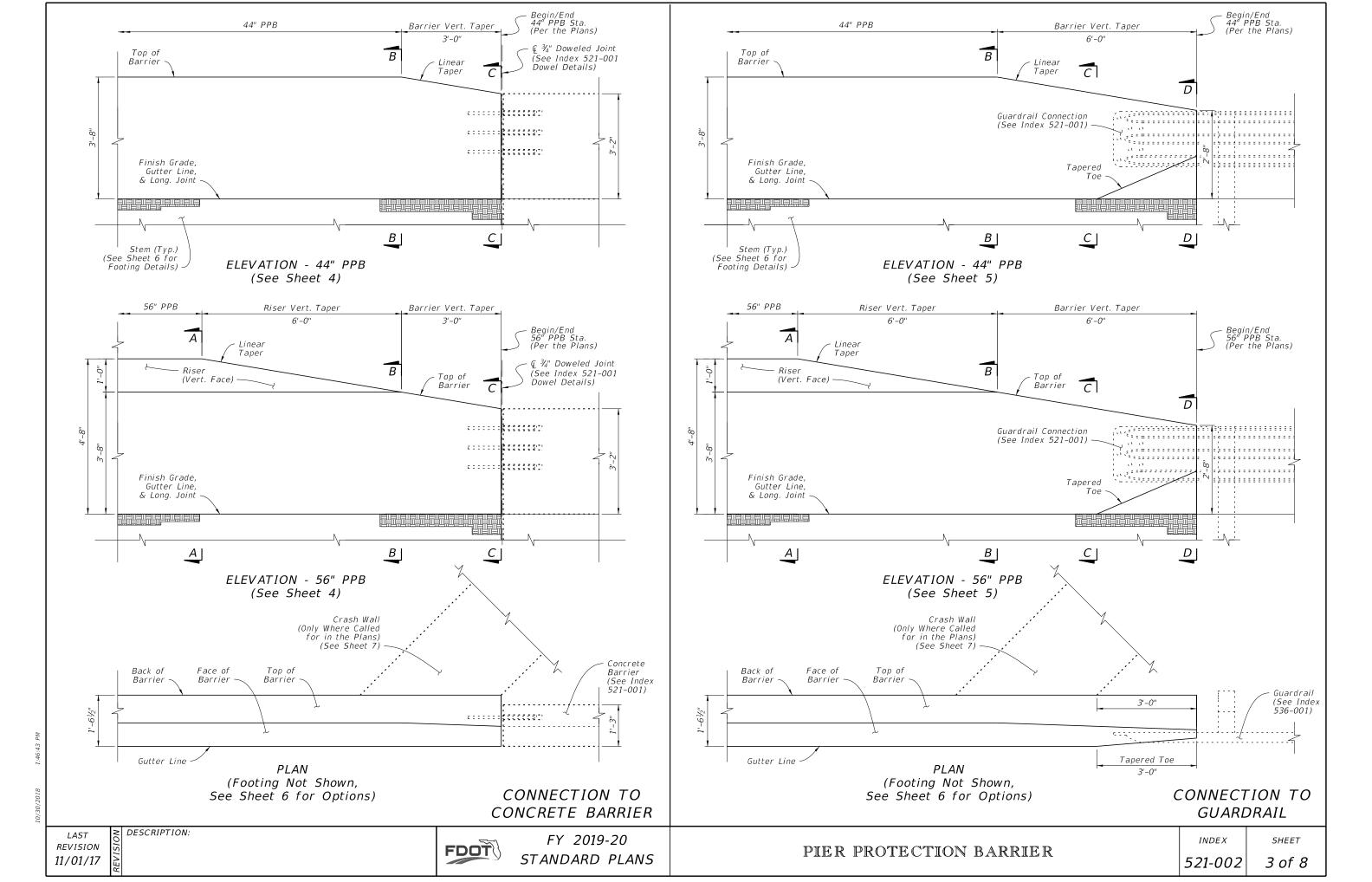
Longitudinal Joints may only be installed where indicated in the following details and notes, with a location tolerance of  $\pm$  1" from the locations shown.

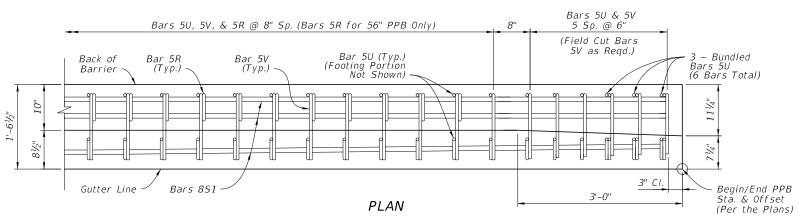
- 3. FOUNDATION: Compact the top 12 inches of the subgrade to at least 98% of the maximum density determined by FM 1-T 180, Method D.
- 4. DRAINAGE INLETS: See Index 425-001 for Shoulder Barrier Inlets, and isolate these structures from Pier Protection Barriers and Footings with 1" Preformed Joint Filler.
- 5. BARRIER END MARKERS: For all free ends of barriers that are not connected to guardrail or concrete barrier, install a Type 3 Object Marker on the end face per Specification 705.
- 6. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification 705. Mount the delineators on the top face of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.
- 7. CRACK CONTROL: Provide 1/2" depth crack control V-Grooves at 15' to 30' spacing. Locate V-Grooves above any joint or discontinuity in the barrier footing. Align V-Grooves perpendicular to the longitudinal axis of the Pier Protection Barrier and make continuous across the top surface and both side faces. For slip formed barriers, score ½" V-Grooves while the concrete is still plastic, otherwise pre-form the joints when stationary forms are utilized.

DESCRIPTION: **REVISION** 11/01/17







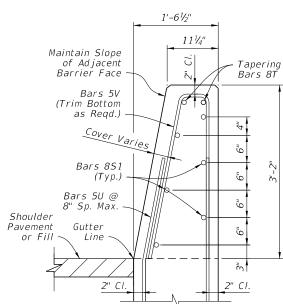


(Details Not Shown Below Gutter Line, See Sheet 6 for Footing and Stem Details) (Only Top & Bottom Longitudinal Steel Shown, See Section Views for All Steel Locations)

#### NOTES:

DESCRIPTION:

- 1. GENERAL: Construct either the 56" PPB or the 44" PPB height as called for in the Plans. See Sheets 2 & 3 for additional plan and elevation details.
- 2. FOOTING OPTIONS: See Sheet 6 for the supporting stem and footing details.



END VIEW C-C (Connects to Adjacent Concrete Barrier, Aligned at Gutter Line)

BARRIER DETAILS - CONNECTION TO CONCRETE BARRIER

LAST **REVISION** 11/01/17

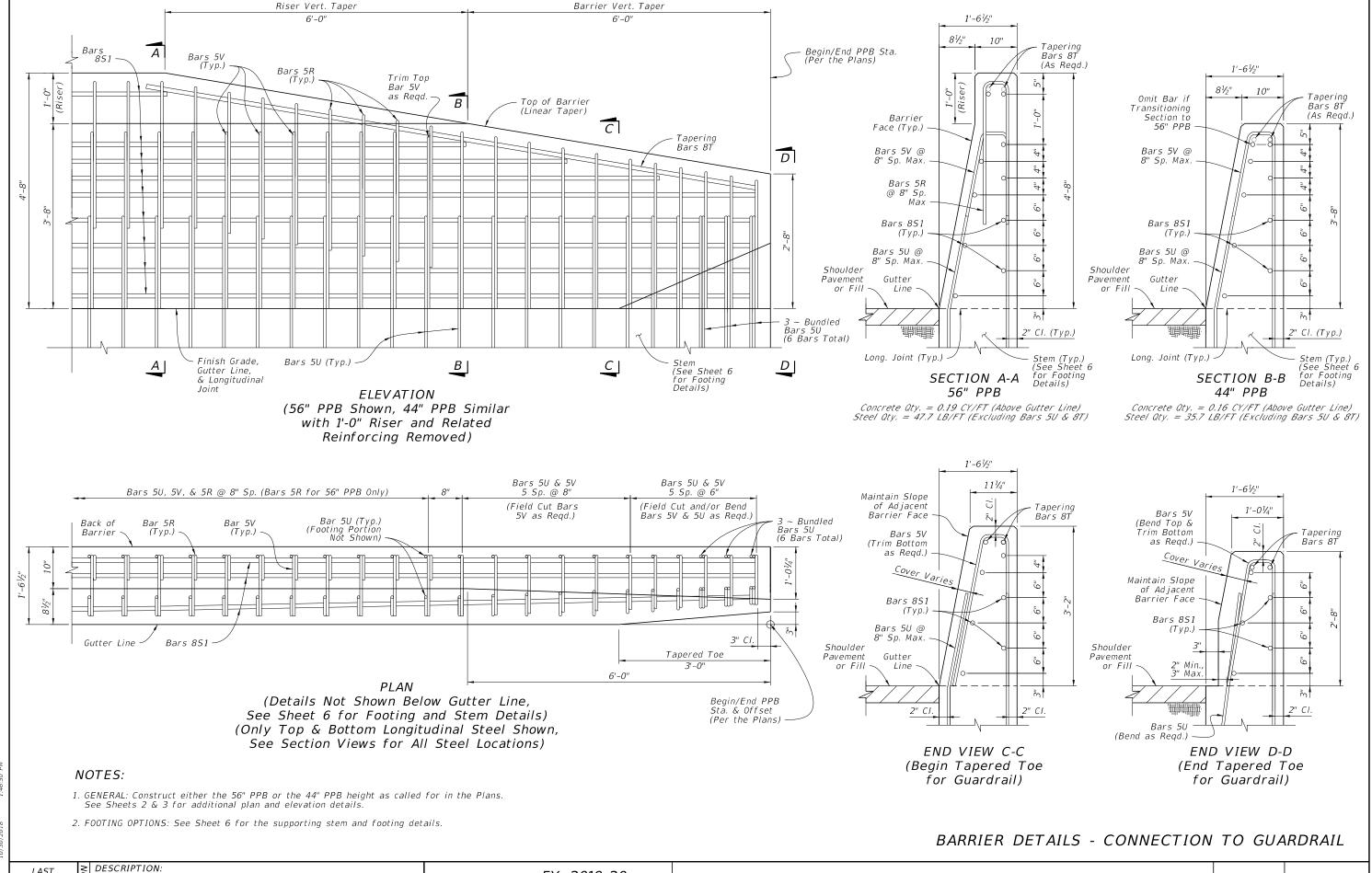
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PIER PROTECTION BARRIER

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**REVISION** 

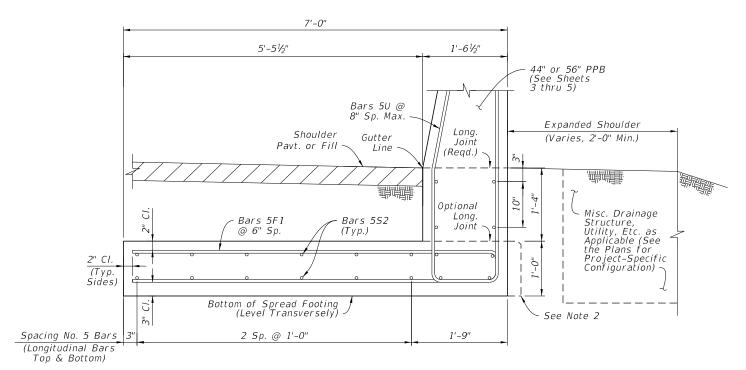
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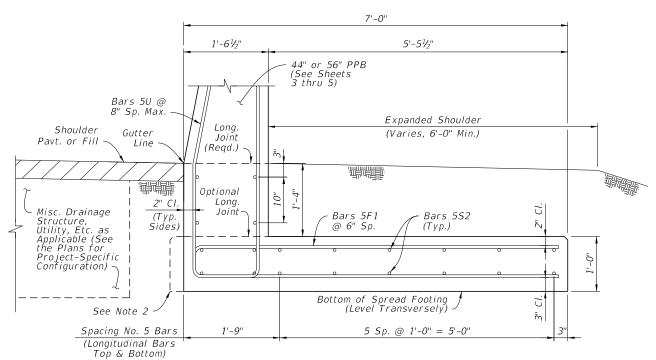
SHEET

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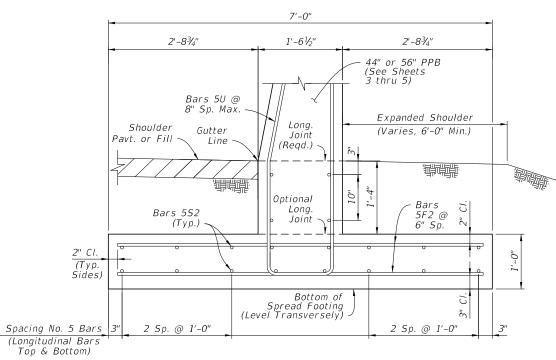
## SECTION E-E REAR-FLUSH FOOTING OPTION

Concrete Qty. = 0.34 CY/FT (Below Gutter Line) Steel Qty. = 63.5 LB/FT (Including Bars 50)



# SECTION G-G FRONT-FLUSH FOOTING OPTION

Concrete Oty. = 0.34 CY/FT (Below Gutter Line) Steel Oty. = 63.5 LB/FT (Including Bars 5U)



# SECTION F-F SYMMETRICAL FOOTING OPTION

Concrete Qty. = 0.34 CY/FT (Below Gutter Line) Steel Qty. = 62.6 LB/FT (Including Bars 50)

#### NOTES:

1. GENERAL: Install the footing options per project-specific requirements, as defined on Sheet 2 and specified per the Plans.

Work with the supported 44" PPB and 56" PPB as shown on Sheets 3, 4, & 5.

- 2. OPTIONAL SLIP FORMING SUPPORT: The 1'-O" depth spread footing may be extended by 3" laterally beyond the face of the stem to provide support for a subsequent slip forming operation above. Do not adjust the steel reinforcement location for the additional concrete.
- 3. GUARDRAIL CONNECTION TAPERED TOE: For tapering the barrier as shown on Sheet 5, View D-D, bend Bars U away from the stem face as required. For this case, the cover requirement is variable for one side of the stem (only at the tapered toe locations).

BARRIER FOOTING OPTIONS

LAST **REVISION** 11/01/17

DESCRIPTION:

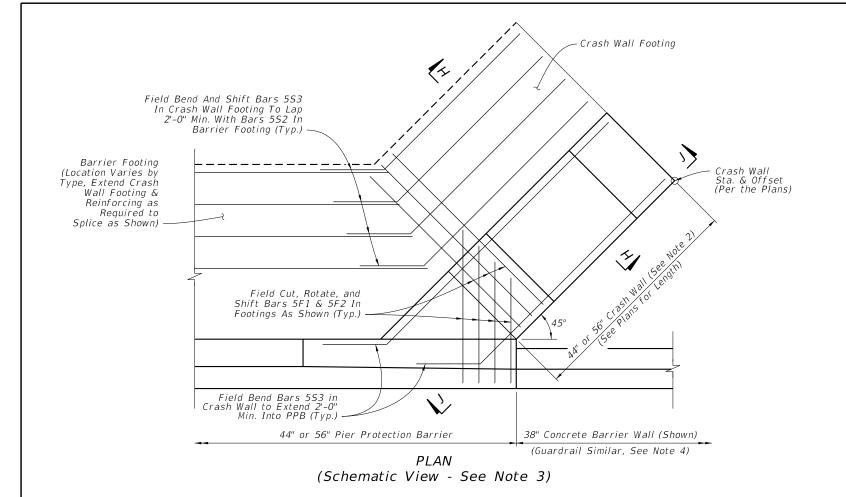
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PIER PROTECTION BARRIER

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Top & Bottom) 44" or 56" Crash Wall (See Note 2) 44" or 56" PPB (See Plans for Length) ∉ Crash Wall (Symmetrical) 2'-6" (Typ.) Match Cross Slope of  $\mathcal{H}$ Shoulder Linear Taper (Typ.)PPB End Height (Тур. Ends) Field Trim Bars 5L and Bend Bars 5S3 Match Cross Slope of Locally As Required Shoulder To Maintain Cover (Typ.)

7'-0" Crash Wall Crash Wall 4'-0" 3'-0" 44" 56" Bars 5E @ 1'-0" Sp. Max. (With Bars 5L) 553 (Typ., Wall & (Typ.)Stem) 3'-8" 1'-0" Bars 5L @ Bars 5L @ @ @ @ 1'-0" Sp. 1'-0" Sp. Sp. Sp. Max. Max. Long. Match Cross Joint Slope of Reqd. Shoulder Optional Bars 5F1 Bars 5S3 Joint @ 6" Sp. (Typ.)2" CI. Spacing Bars 5S3 (Typ. Sides) (Longitudinal Bars Each Face) Bottom of Spread Footing (Level Transversely) See Note 5 Spacing Bars 5S3 3 Sp. @ 1'-0" (Longitudinal Bars

> SECTION H-H CRASH WALL Concrete Qty. = 0.82 CY/FT (44" Crash Wall) or 0.93 CY/FT (56" Crash Wall)

NOTES:

1. GENERAL: Only where called for in the Plans, install the Crash Wall as a supplement for PPB. If applicable, see the Plans for the corresponding Station and Offset required.

Steel Qty. = 71.8 LB/FT (44" Crash Wall) or 76.0 LB/FT (56" Crash Wall)

For additional layout details, see Sheets 2 & 3.

- 2. CRASH WALL HEIGHT: Install the Crash Wall at a height which matches the adjacent PPB (either 44" or 56").
- 3. SCHEMATIC VIEWS: Only partial reinforcing is shown in the Schematic Views to establish a trend while keeping clarity. For all reinforcing steel locations and spacing requirements, see Section H-H.
- 4. GUARDRAIL CONNECTIONS: To facilitate guardrail connections, shift the Crash Wall 3 feet from the end of the PPB as shown on Sheets 2 & 3.
- 5. OPTIONAL SLIP FORMING SUPPORT: The 1'-0" depth spread footing may be extended by 3" laterally beyond the face of the wall to provide support for a subsequent slip forming operation above. Do not adjust the steel reinforcement location for the additional concrete.

VIEW J-J CRASH WALL ELEVATION (Schematic View - See Note 3)

CRASH WALL DETAILS

**REVISION** 11/01/17

DESCRIPTION:

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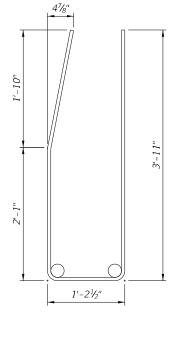
FY 2019-20 STANDARD PLANS

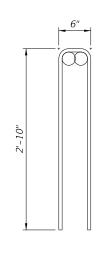
INDEX

SHEET

BILL OF REINFORCING STEEL					
MARK	SIZE	LENGTH			
V	5	7'-5"			
U	5	8'-11"			
R	5	6'-0"			
F1	5	13'-9"			
F2	5	Varies (Straight)			
L	5	6'-5" / 7'-5"			
E	5	4'-6"			
S1	8	Varies (Straight)			
<i>S2, S3</i>	5	Varies (Straight)			

-	1'-21/2"	
81/8" 63/8"		
3-6"		





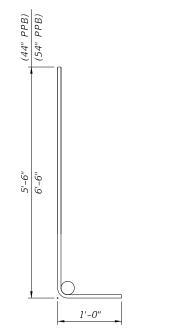
BARS 5R

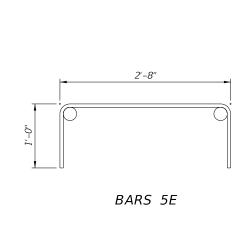
BARS 5V

NOTES:

- 1. Work with the Standard Bar Bending Details per Index 415-001.
- 2. All bar dimensions in the bending diagrams are out to out.

BARS 5U





BARS 5F1

BARS 5L

BAR BENDING DIAGRAMS

REVISION 11/01/17

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≥ DESCRIPTION:

#### GENERAL NOTES:

- 1. GENERAL: Construct Opaque Visual Barrier (OVB) in accordance with Specification 521, and use either cast in place or precast panels. Do not cast OVB concrete monolithically with the Concrete Barrier or Traffic Railing; use an ASTM D6380, Class S, Type III Organic Felt bond breaker as needed.
- 2. DOWEL BAR CONNECTION: For the embedment in Concrete Barrier or Traffic Railing concrete, dowel bars must be either cast in place for new concrete or grouted in place for existing concrete. Embed the dowel bars to the corresponding depths shown, and use the bar lengths provided in the Dowel Bar Length Table.

At cast in place embedment locations, longitudinally shift the dowel bars only as required to avoid reinforcing steel in the Concrete Barrier or Traffic Railing.

At grouted embedment locations, drill % 0 holes to a depth of  $6\frac{1}{4}$ ". Use only approved non-shrink grout on the APL. Drilling through existing reinforcing steel is permitted.

3. TRANSVERSE JOINTS: Place 1/2" Transverse Joints with a maximum spacing of 50'-0" and a minimum spacing of 20'-0". Use a consistent spacing where practical.

Without violating the above spacing requirements, place Transverse Joints matching the location and width of open joints in the supporting Concrete Barrier or Traffic Railing.

- 4. SLOPED END TREATMENTS: Regardless of the traffic direction, place Sloped End Treatments on all exposed ends of OVB, excluding leave-outs for barrier-mounted signs and light poles. See Note 7 below.
- 5. BARRIER-MOUNTED SIGNS AND LIGHT POLES: Where signs and barrier-mounted light pole structures conflict with placement of OVB, end and restart the OVB with a transverse vertical face located a longitudinal distance of 2"  $(\pm \frac{1}{2})$ " from the base of the structure. Follow the same reinforcing scheme and concrete cover requirement for the Transverse Joint shown herein. See Note 7 below
- 6. LARGE BARRIER-MOUNTED SIGN SUPPORTS: See Sheet 2 for details. See Note 7 below.
- 7. LEAVE-OUTS: OVB leave-outs are longitudinal gaps in OVB segments required to accommodate barrier-mounted signs and light pole placement. Leave-outs up to 15 feet in length are included in OVB length measurement.
- 8. ASYMMETRICAL CONCRETE BARRIER SECTIONS: When mounting on top of an asymmetrical Concrete Barrier section (not shown), align the centerline of the OVB with the centerline of the top face of the Concrete Barrier section.
- 9. SPLIT CONCRETE BARRIER SECTIONS: For split Concrete Barrier sections that run separately (for vertical structures, bridges, etc.), OVB is only required on top of one of the Concrete Barrier sections. Place OVB on top of the Concrete Barrier section with the highest elevation. Longitudinally overlapping OVB runs are permitted where called for in the Plans, which are designated with overlapping Begin and End Station OVB callouts.
- 10. VERTICAL REINFORCING: Place vertical No. 3 bars with the spacing shown, except that No. 3 bars at the dowel bar locations may be shifted longitudinally to fit or they may be omitted at the option of the
- 11. OPTIONAL WELDED WIRE REINFORCEMENT: With the approval of the Engineer, the No. 3 bars shown herein may be replaced with welded wire reinforcement in accordance with Specification 415. Use welded wire reinforcement of equal or greater strength than the bars being replaced; maintain the same cover requirements with equivalent or smaller spacing
- 12. VARIABLE HEIGHT CONCRETE BARRIERS: See Sheet 2 for details.

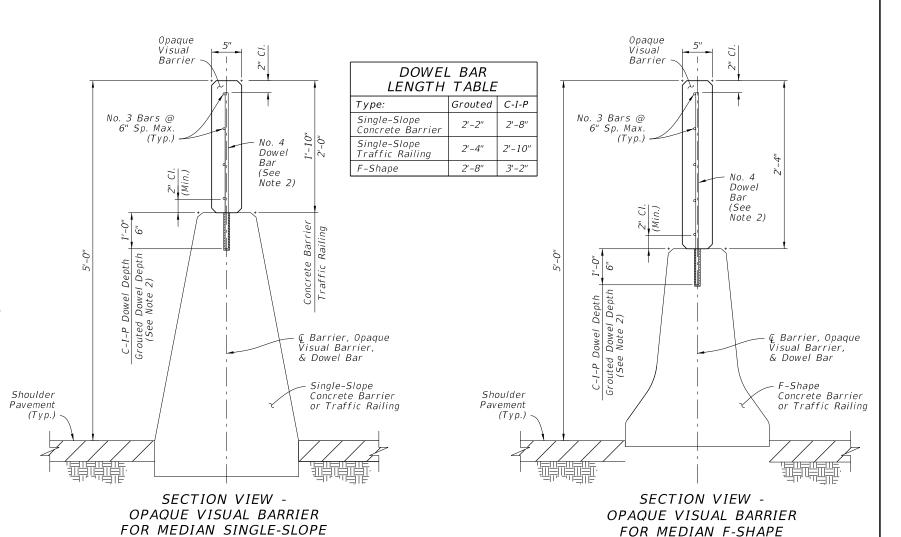
DESCRIPTION:

13. CONCRETE BARRIER AND TRAFFIC RAILING TRANSITIONS BETWEEN DIFFERING SECTIONS: Transition the OVB section using a method similar to the OVB Linear Bottom Transition shown in Elevation View 'B' on Sheet 2, except adjust the longitudinal length of the transition as required.

## ELEVATION VIEW - OPAQUE VISUAL BARRIER

CONCRETE BARRIER

OR TRAFFIC RAILING



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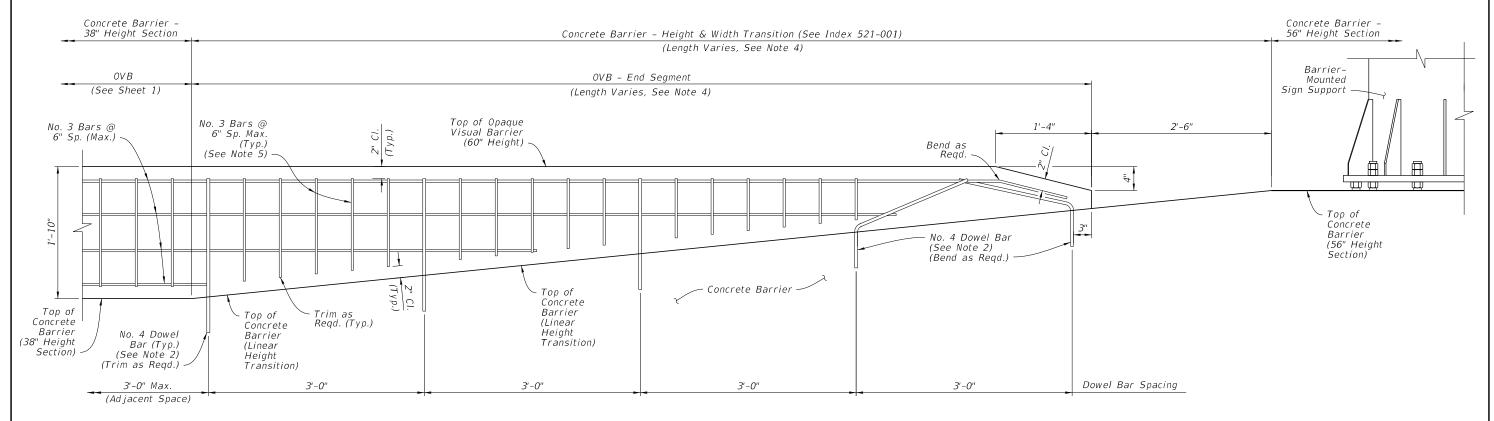
OPAQUE VISUAL BARRIER

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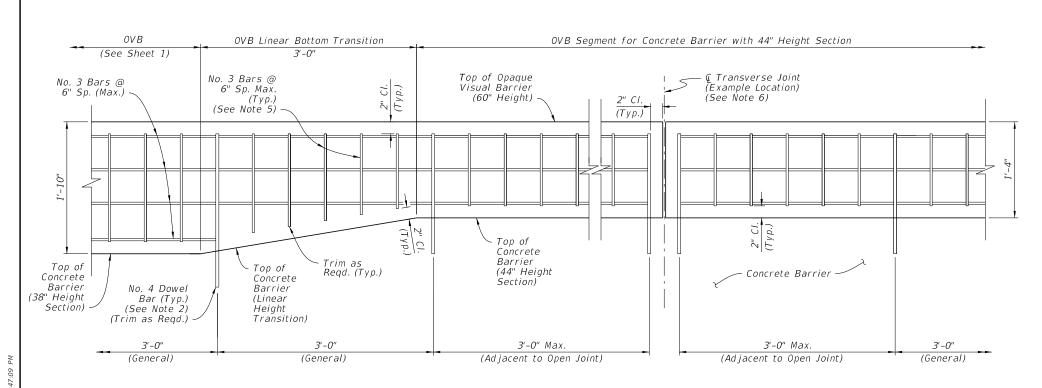
CONCRETE BARRIER

OR TRAFFIC RAILING

SHEET 1 of 2



# ELEVATION VIEW 'A' - OVB END SEGMENT AT CONCRETE BARRIER HEIGHT TRANSITION FROM 38" HEIGHT TO 56" HEIGHT SECTION (REVERSE DIRECTION SIMILAR BY OPPOSITE HAND)



# ELEVATION VIEW 'B' - OVB SEGMENT FOR CONCRETE BARRIER WITH 44" HEIGHT SECTION (OVB LINEAR BOTTOM TRANSITION SHOWN, REVERSE DIRECTION SIMILAR BY OPPOSITE HAND)

# NOTES:

- 1. LATERAL DIMENSIONS: Maintain the OVB section width and lateral placement as defined on Sheet 1.
- 2. DOWEL BAR LENGTHS & CONNECTIONS: For the differing OVB section heights, trim or adjust the dowel bar lengths as required to meet the clearances shown while maintaining the dowel bar connection requirements of Sheet 1.

Elevation View 'A' - For the two dowel bars closest to the OVB end location, use full dowel bar lengths and bend as shown to maintain clearances. Overlapping dowel bars may deviate from the lateral centerline as required.

3. DOWEL BAR SPACING:

Elevation View 'B' - The dowel locations shown in this detail are examples only, and may shift to maintain the spacing pattern that is governed by adjacent OVB. Maintain the dowel bar spacing scheme as defined on Sheet 1; place dowel bars within the OVB Linear Bottom Transition as required.

4. SEGMENT LENGTHS:

Elevation View 'A' - The length of the OVB End Segment is governed by the length of linear width and height transition of the Concrete Barrier.

Elevation View 'B' - The length of the reduced-section OVB segment is governed by the length of Concrete Barrier with 44" Height Section.

- 5. VERTICAL REINFORCING: For the differing OVB section heights, trim or adjust the vertical No. 3 Bar lengths as required to meet the clearances shown.
- 6. TRANSVERSE JOINTS:

Follow the requirements of Sheet 1.

Elevation View 'A' - Do not place Transverse Joints within the End Segment.

Elevation View 'B' - Maintain the Transverse Joint spacing scheme as defined on Sheet 1; place dowel bars within the OVB Linear Bottom Transition as required.

REVISION 11/01/18

DESCRIPTION:

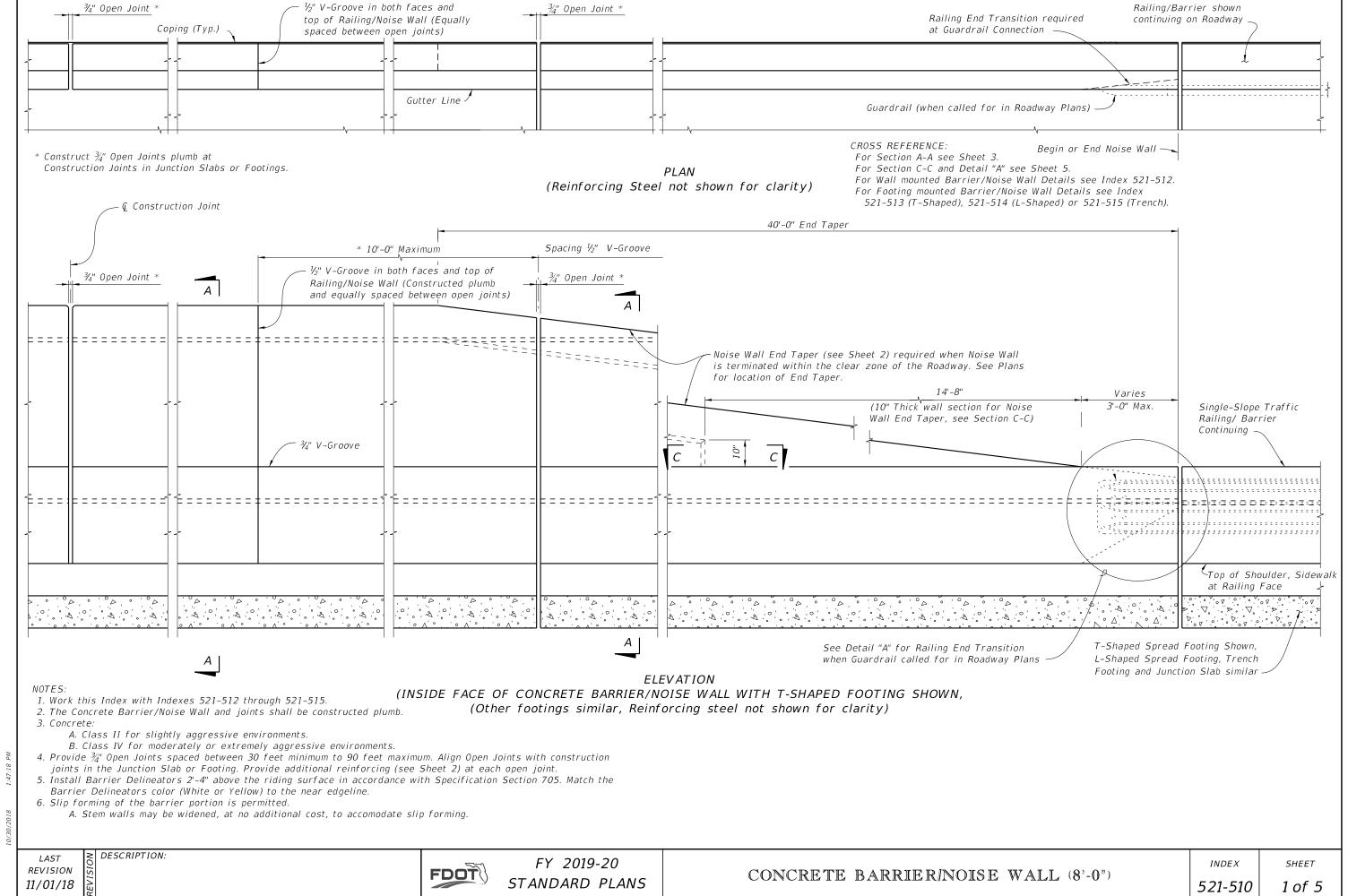
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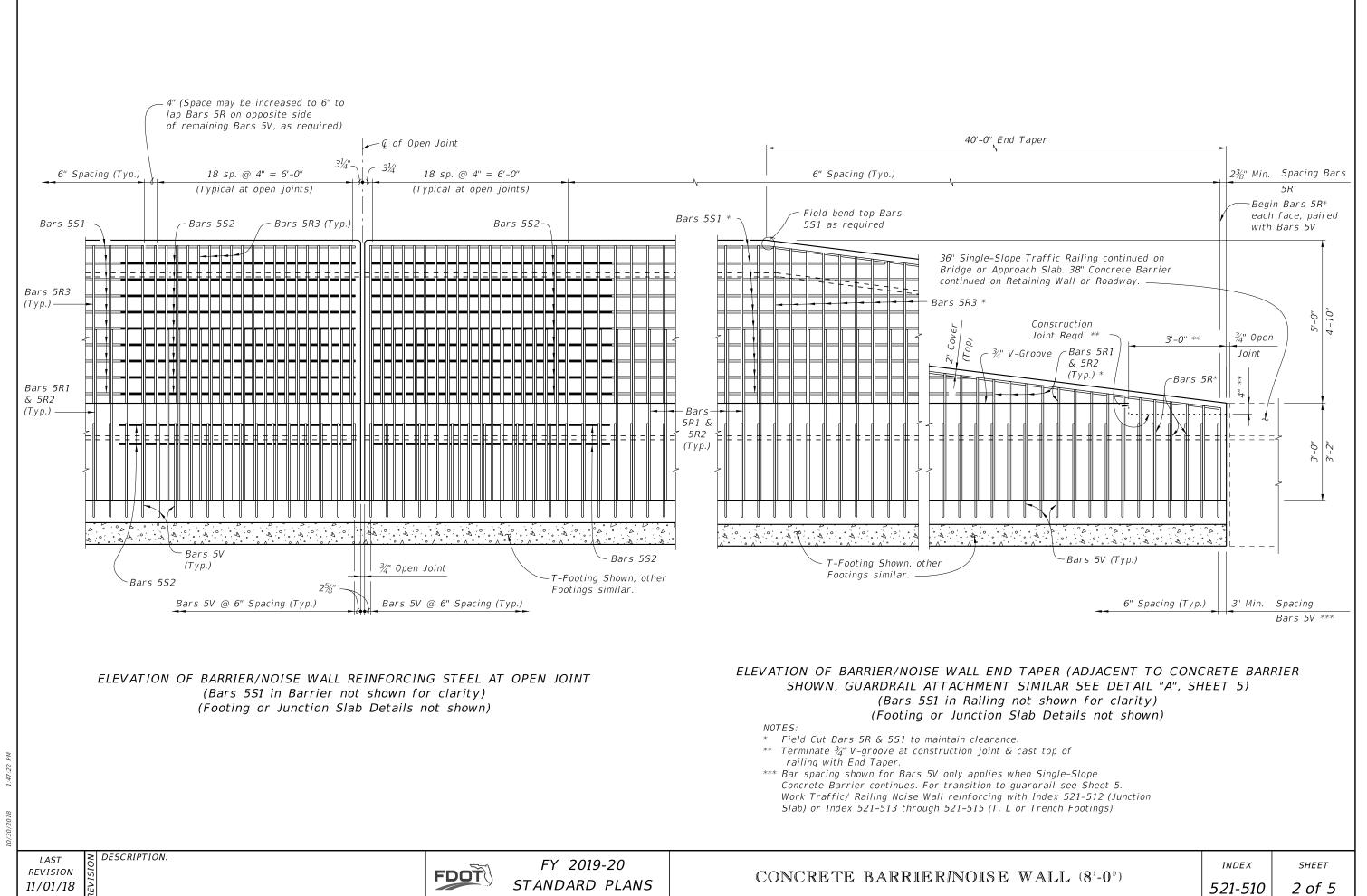
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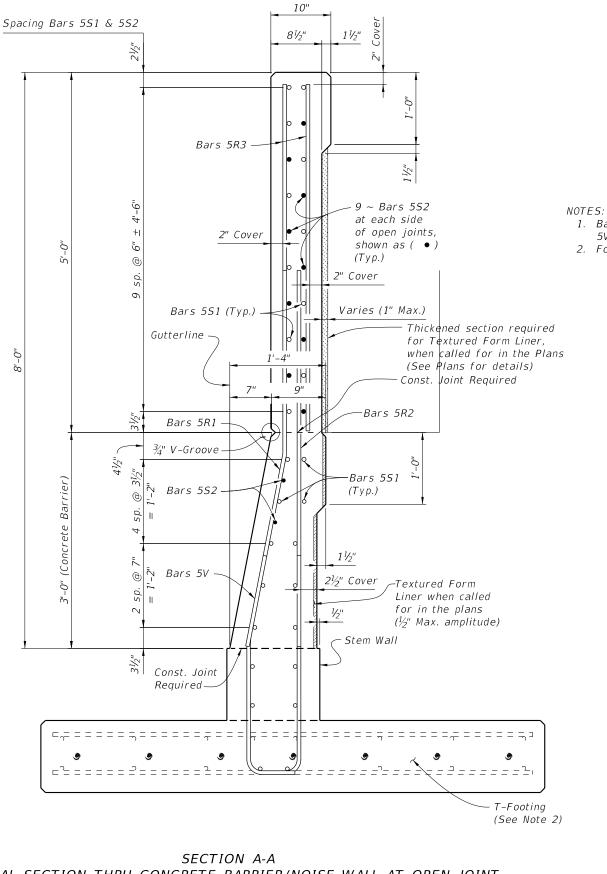
OPAQUE VISUAL BARRIER

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SHEET







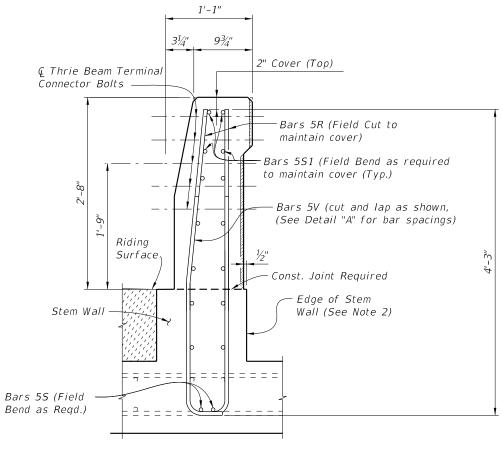
TYPICAL SECTION THRU CONCRETE BARRIER/NOISE WALL AT OPEN JOINT (Section Thru T-Footing Shown, Section Thru Junction Slab, L or Trench Footings similar)

CROSS REFERENCE:

For locations of Section A-A see Sheet 1. For location of View B-B, see Sheet 5. For Detail "A", see Sheet 5

1. Bars 5V shown are for T-Shape footings. 5V for Junction Slab, L-Shape and Trench footings are similar.

2. Foundation Details: Index 521-512 (Junction Slab) Index 521-513 (T-Shape) Index 521-514 (L-Shape) Index 521-515 (Trench)



VIEW B-B END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (T-Footing shown, Junction Slab, L or Trench Footings similar)

REVISION 11/01/18

DESCRIPTION:

FDOT

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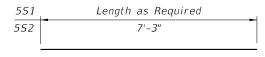
SHEET 3 of 5

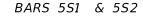
*521-510* 

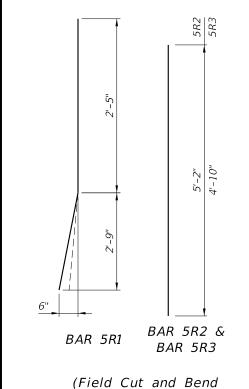
ESTIMATED TRAFFIC					
RAILING/NOISE WALL QUANTITIES					
<u> ITEM</u>	UNIT	QUANTITY			
Concrete (Railing)	CY/LF	0.107			
Concrete (Noise Wall)	CY/LF	0.136			
Reinforcing Steel (Typical)	LB/LF	69.36			
Additional Reinf. @ Open Joint	LB	226.85			

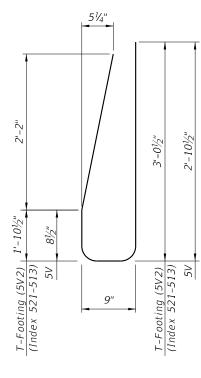
(The above quantities are based on the Concrete Barrier/ Noise wall typical section, (excluding junction slab or footing)

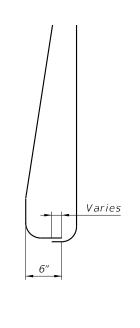
#### REINFORCING STEEL BENDING DIAGRAMS BILL OF REINFORCING STEEL MARK SIZE LENGTH R1 5 5'-2" 5 5'-2½" R2 R3 5 4'-10" 51 5 As Regd. 52 5 7'-3" 5 V (Wall) 6'-6½" 5 (T-Footing) 7'-8½"











REINFORCING STEEL NOTES:

for Railing End Transition)

STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut (Railing End Transition)

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.
- 4. Bars 5S1 may be continuous or spliced at the construction joints. Lap splices for Bars 5R, 5S1 and 5W shall be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.
- 6. See Index 521-514 and 521-515 for L-shaped and Trench footing vertical reinforcing.

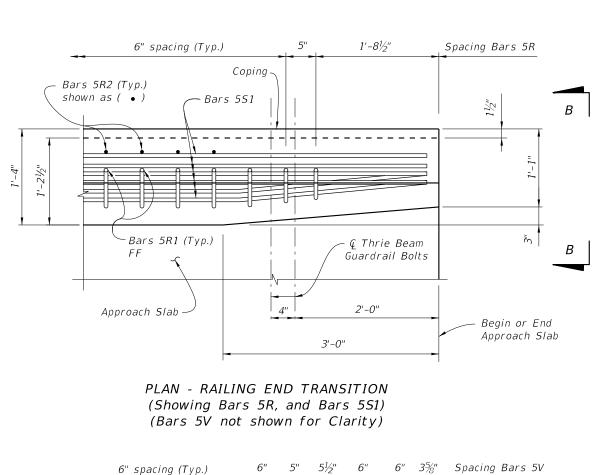
CROSS REFERENCE: See Index 521-512 for Junction Slab Details and Indexes 521-513 thru 521-515 for additional footing details.

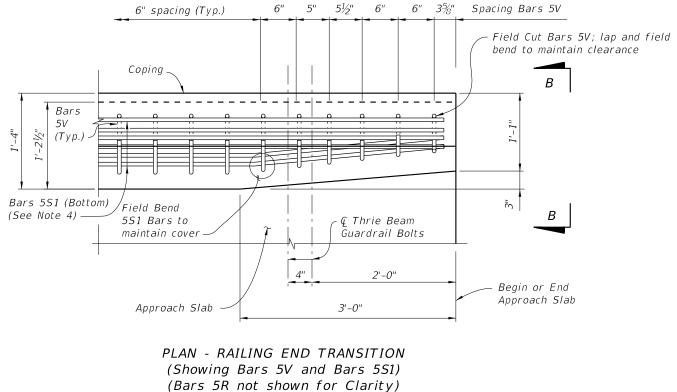
**REVISION** 11/01/18

DESCRIPTION:

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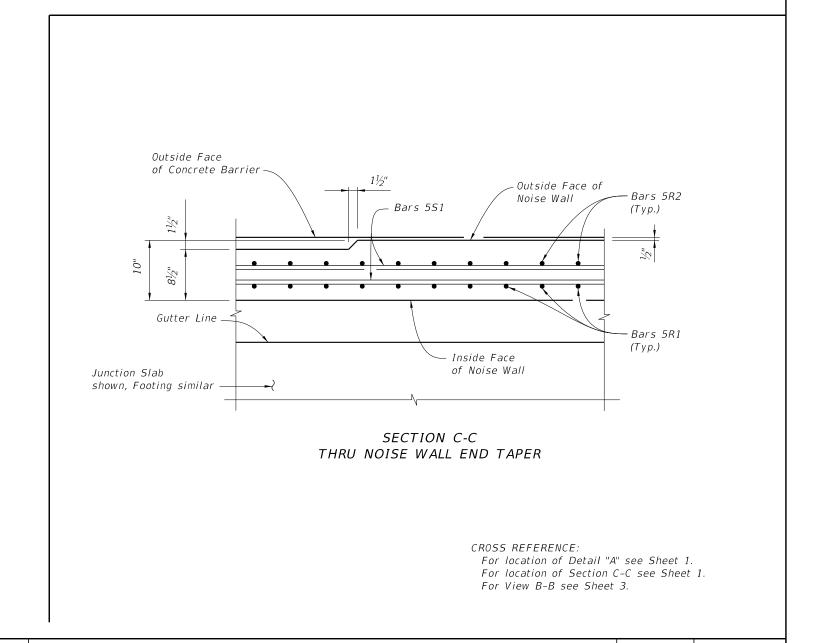




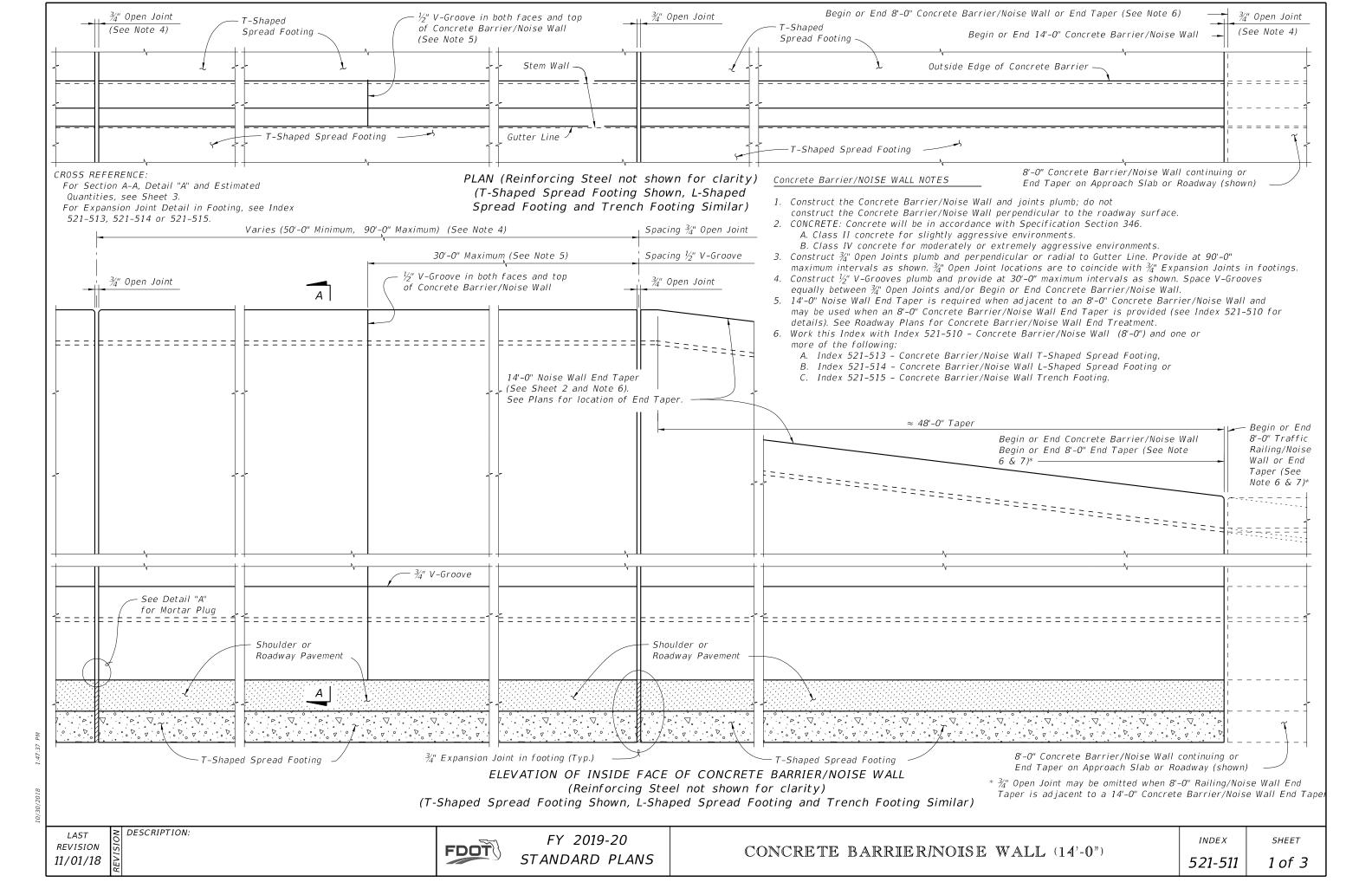
= DETAIL "A" ====

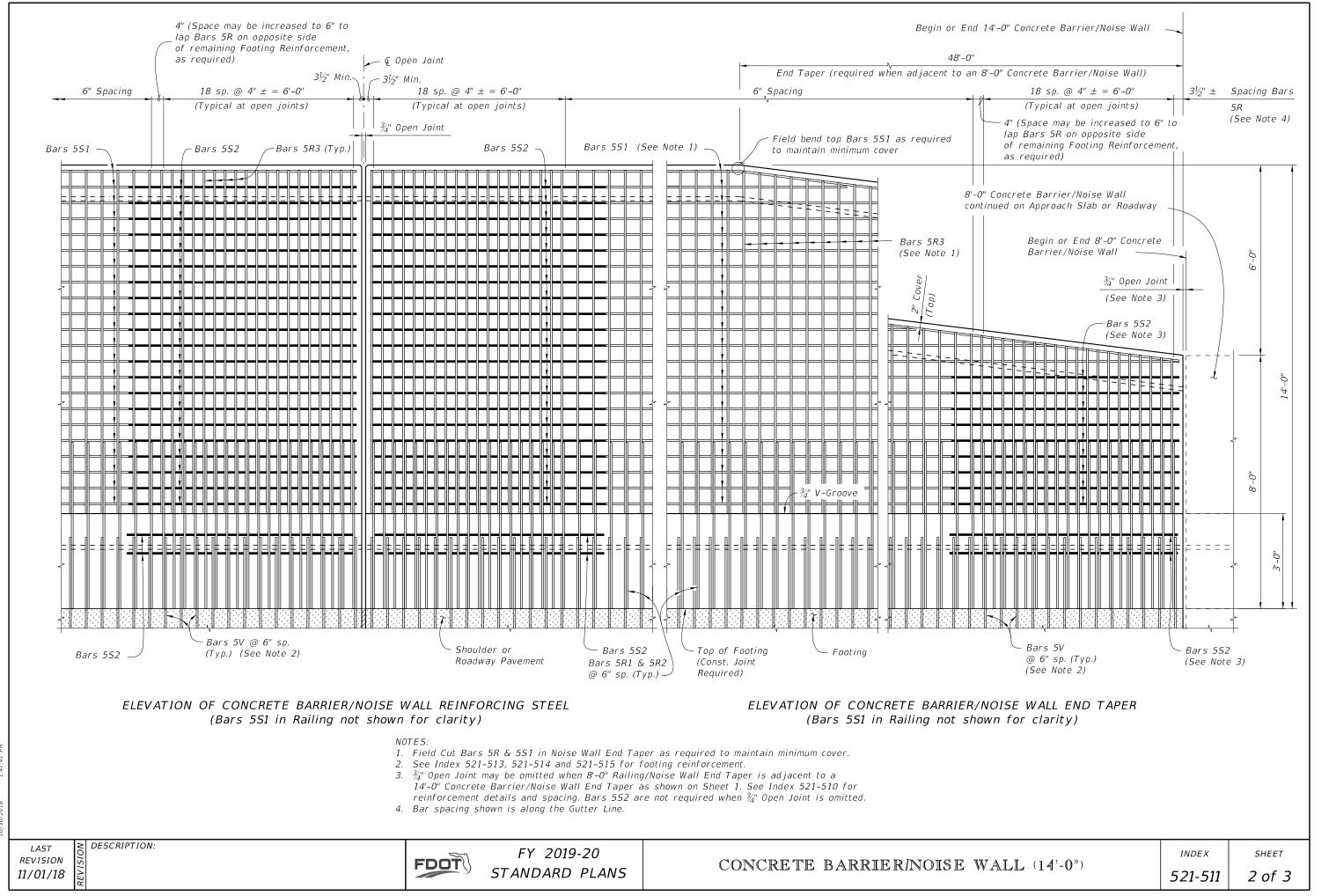
#### DETAIL "A" NOTES:

- 1. Begin placing Railing Bars 5V at the railing end and proceed toward the guardrail (thrie beam) terminal connector to ensure placement of guardrail bolt holes. Pair Bars 5R with Bars 5V as shown. Clearance of Bars 5R & 5V to guardrail bolt holes shall be checked to prevent cutting of bars if bolt holes are to be drilled. Shift bars locally where conflicts occur.
- 2. For Guardrail connection details see Index 536-001.
- 3. Omit Railing End Transition if a Single-Slope Concrete Barrier/ Barrier continues beyond the End Taper. See the Plan Sheets.
- 4. Field cut Bars 5R2 to maintain cover. Field cut Bars 5V and lap as necessary to maintain cover; field cut & bend Bars 5R1 front leg (more plumb) to maintain cover and tie to S1 Bars. (See Sheet 1 Notes 1 and 2)

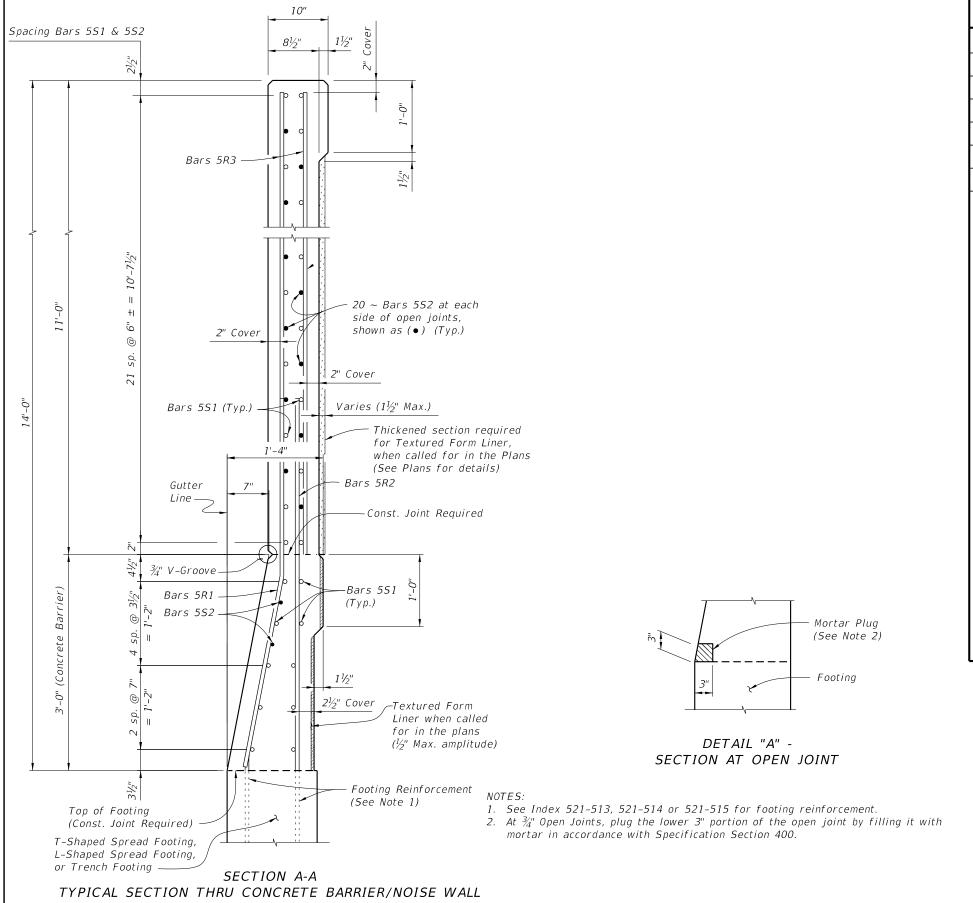


DESCRIPTION:



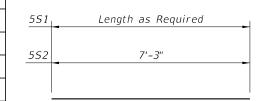


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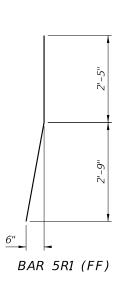


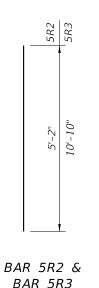
## REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEE				
MARK	SIZE	LENGTH		
R1	5	5'-2"		
R2	5	5'-2 ¹ / ₂ "		
R3	5	10'-10"		
51	5	AS REQD.		
52	5	7'-3"		



BARS 5S1 & 5S2





## REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Bars 5R may be continuous or spliced at construction joints. Lap splices for Bars 5R, and 5S1 will be a minimum of 2'-2".
- 4. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

# ESTIMATED CONCRETE BARRIER/NOISE WALL QUANTITIES

ITEM	UNIT	QUANTITY
Concrete (Concrete Barrier)	CY/FT	0.107
Concrete (Noise Wall, excluding any thickening)	CY/FT	0.293
Reinforcing Steel (Railing/Noise Wall) (Bars R1, R2, R3, S1 & V)	LB/FT	100.31
Additional Reinf. @ Open Joint (Railing/Noise Wall)	LB	397.38

For locations of Section A-A and Detail "A", see Sheet 1.

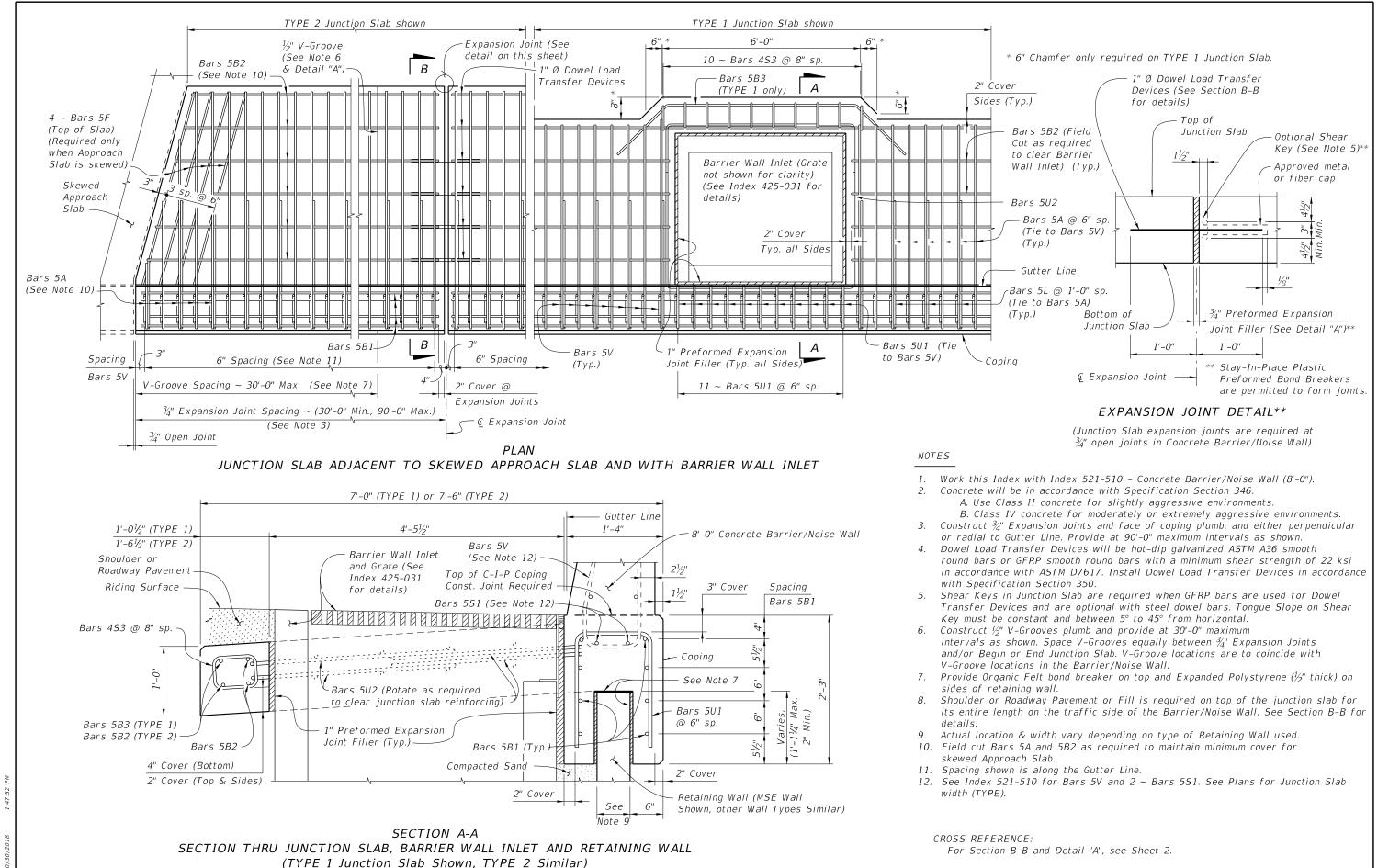
**REVISION** 11/01/18

DESCRIPTION:



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LAST REVISION 11/01/18

DESCRIPTION:

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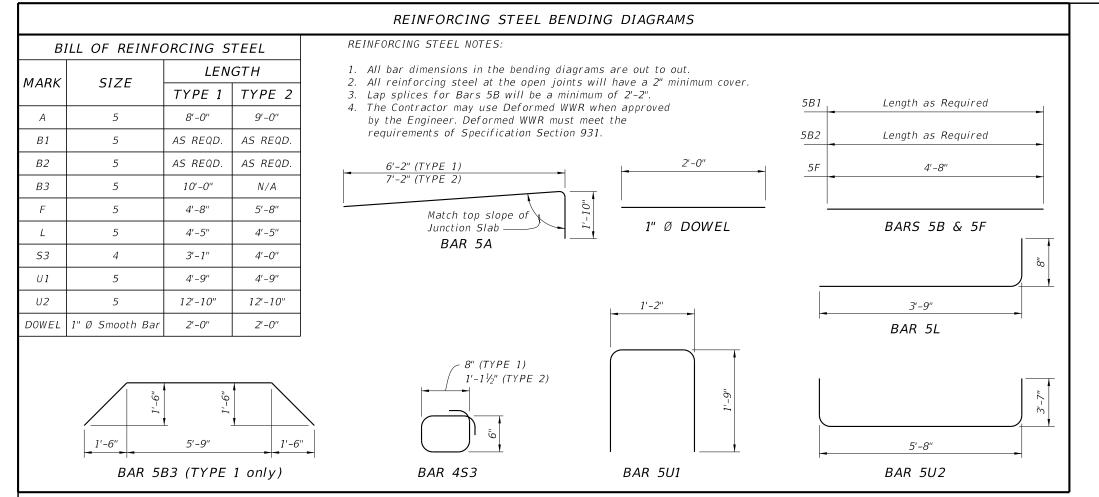
CONCRETE BARRIER/NOISE WALL (8'-0")
JUNCTION SLAB

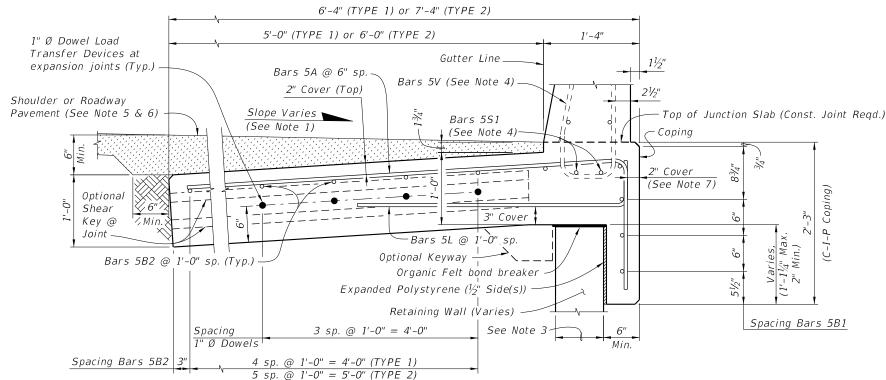
INDEX

SHEET

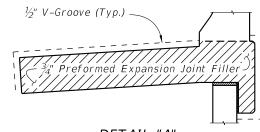
521-512

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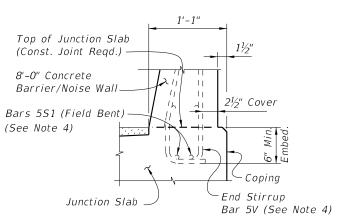


SECTION B-B TYPICAL SECTION THRU JUNCTION SLAB AND RETAINING WALL



DETAIL "A"

(Showing Locations of ½" V-Grooves and 3/4" Preformed Expansion Joint Filler)



PARTIAL END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V and Bars 5B1)

NOTE: See Index 521-510. Detail "A" for details.

#### ESTIMATED JUNCTION SLAB QUANTITIES QUANTITY ITEM UNIT TYPE 1 TYPE 2 0.305 CY/FT 0.268 Concrete (Junction Slab) Reinforcing Steel (Typical) LB/FT 31.72 34.85 Additional Reinf. @ Expansion Joint LB 21.36 21.36

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. Vary Junction Slab slope based on roadway cross slope to maintain a minimum 6" asphalt depth at the edge of the slab as shown.
- 3. Actual width varies depending on type of Retaining Wall used.
- 4. See Index 521-510 for Bars 5V and Bars 5S1.
- 5. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finished grade.
- 6. For Asphalt: Shoulder or Roadway Pavement will be SuperPave Structural asphalt. Variable thickness asphalt will be structural overbuild.
- 7. If slip forming is used, submit shop drawings for approval showing Expansion Joint support details and 21/3" side cover with adjusted Typical Section dimensions.

CROSS REFERENCE:

For location of Section B-B, see Sheet 1.

REVISION 11/01/18

DESCRIPTION:

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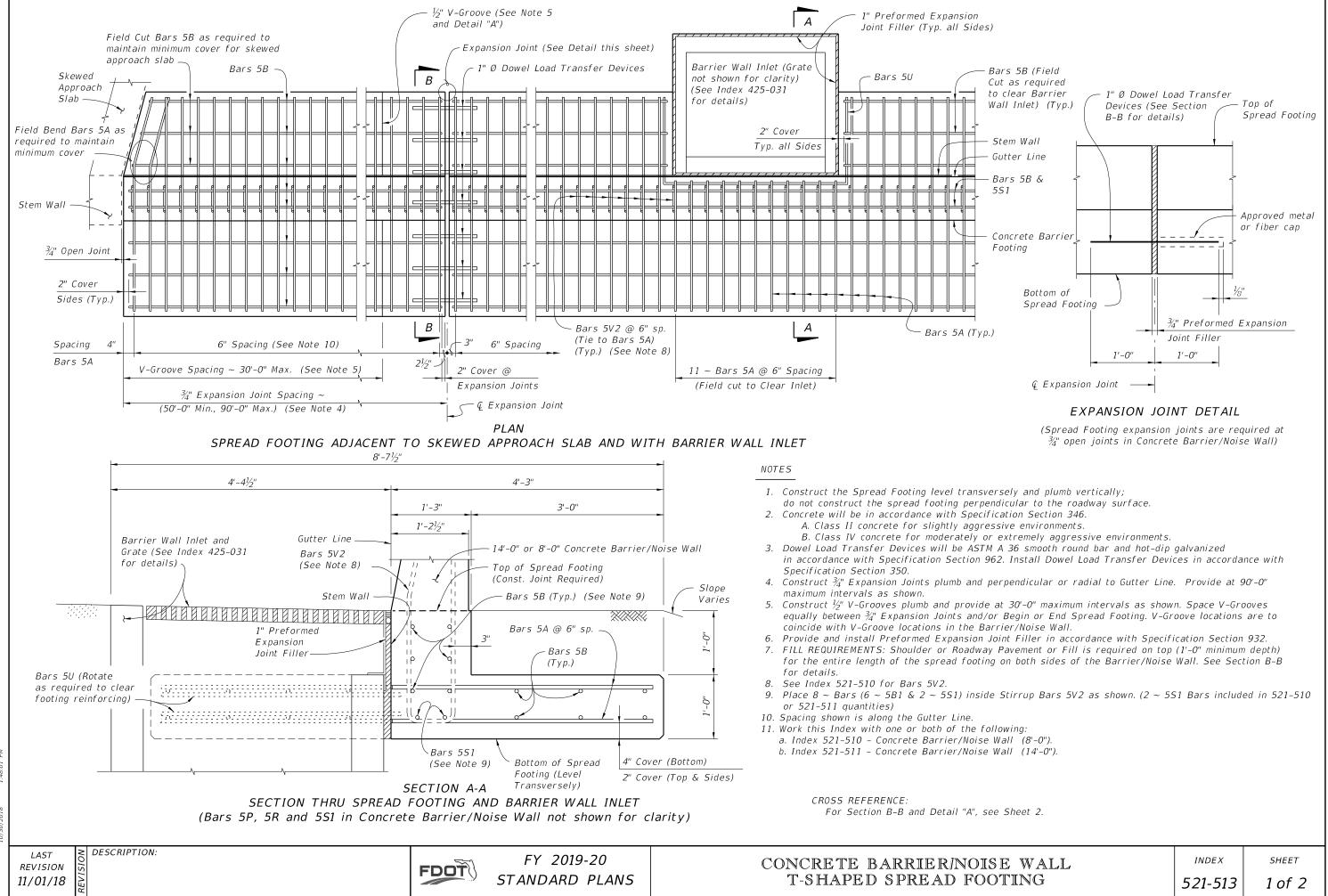
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CONCRETE BARRIER/NOISE WALL (8'-0")

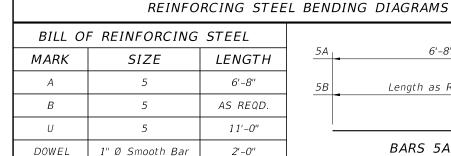
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JUNCTION SLAB

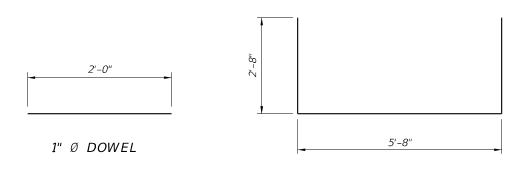


10/30/2018



5A	6'-8"	
5B	Length as Required	

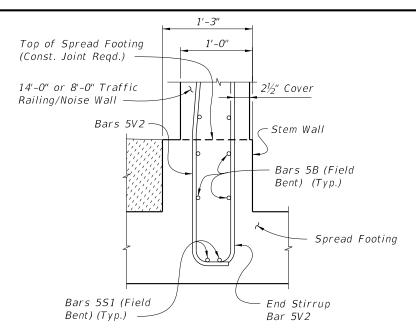
BARS 5A & 5B



BAR 5U

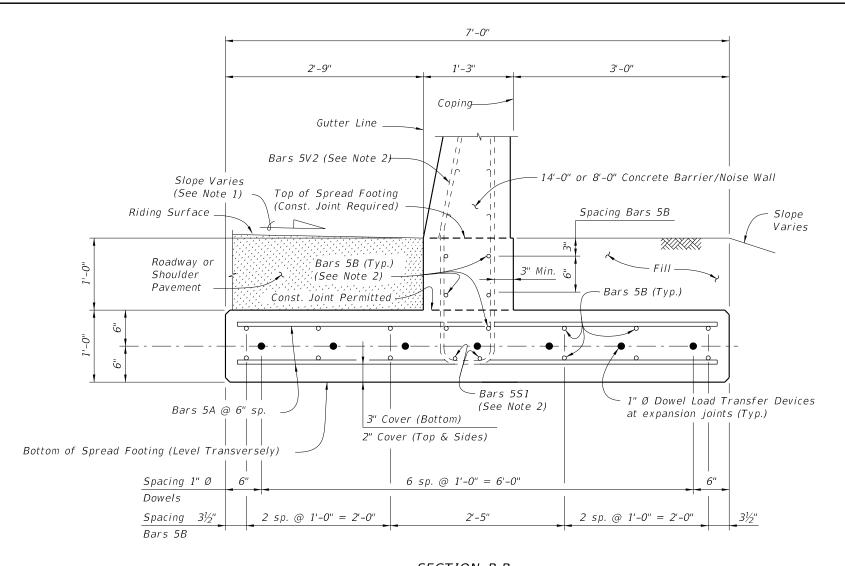
### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-2".
- 4. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



PARTIAL END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V2, and Bars 5B inside of Stirrup Bars 5V2)

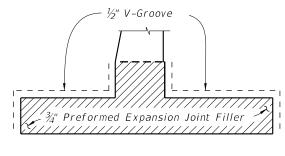
NOTE: See Index 521-510, Detail "A" for details.



# SECTION B-B TYPICAL SECTION THRU SPREAD FOOTING (Bars 5R and 5S1 in Concrete Barrier/Noise Wall not shown for clarity)

#### NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. See Sheet 1, Notes 8 & 9.



# DETAIL "A"

(Showing Locations of ½" V-Grooves and 3/4" Preformed Expansion Joint Filler)

ESTIMATED T-SHAPED SPREAD FOOTING QUANTITIES   			
ITEM	UNIT	QUANTITY	
Concrete (Footing)	CY/FT	0.312	
Reinforcing Steel (Typical)	LB/FT	25.90	
Additional Reinf. @ Expansion Joint	LB	37.38	

Note: The reinforcing steel quantity includes the difference between Index 521-510 or 521-511 and Bars 5V shown. Bars 5S1 are included in Index 521-510 or 521-511 quantities.

## CROSS REFERENCE:

For location of Section B-B, see Sheet 1.

**REVISION** 11/01/17

DESCRIPTION:

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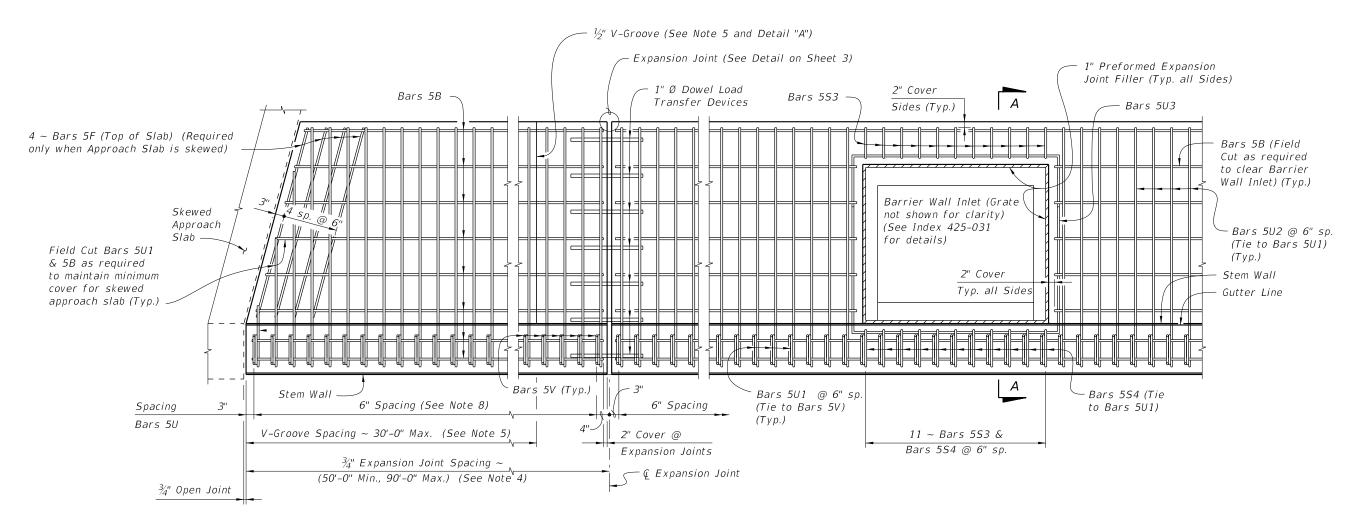
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CONCRETE BARRIER/NOISE WALL T-SHAPED SPREAD FOOTING

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# PLAN - OPTION B SPREAD FOOTING ADJACENT TO SKEWED APPROACH SLAB AND WITH BARRIER WALL INLET (Option A Similar) (Bars S1 Not Shown)

# NOTES

- 1. Construct the Spread Footing level transversely; do not construct the spread footing perpendicular to the roadway surface.
- 2. Concrete will be in accordance with Specification Section 346.
  - A. Class II concrete for slightly aggressive environments.
  - B. Class IV concrete for moderately or extremely aggressive environments.
- 3. Dowel Load Transfer Devices will be ASTM A 36 smooth round bar and hot-dip galvanized in accordance with Specification Section 962. Install Dowel Load Transfer Devices in accordance with Specification
- 4. Construct ¾" Expansion Joints plumb and perpendicular or radial to Gutter Line. Provide at 90'-0" maximum intervals as shown.
- 5. Construct ½" V-Grooves plumb and provide at 30'-0" maximum intervals as shown. Space V-Grooves equally between 3/4" Expansion Joints and/or Begin or End Spread Footing, V-Groove locations are to coincide with V-Groove locations in the Concrete Barrier/Noise Wall.
- 6. Provide and install Preformed Expansion Joint Filler in accordance with Specification Section 932.
- 7. Shoulder or Roadway Pavement and Fill is required on the traffic side of the spread footing for a distance of 4'-0" and the full length of the spread footing (3'-0" minimum depth) on the backside of the spread footing for Option A. Fill is required for a distance of 4'-0" on the backside of the spread footing and the full length of the spread footing (3'-0" minimum depth) on the traffic side of the spread footing for Option B. See Typical Sections on Sheets 2 and 3 for details.
- 8. Spacing shown is along the Gutter Line.
- 9. Work this Index with one or both of the following:
  - a. Index 521-510 Concrete Barrier/Noise Wall (8'-0").
- b. Index 521-511 Concrete Barrier/Noise Wall (14'-0").

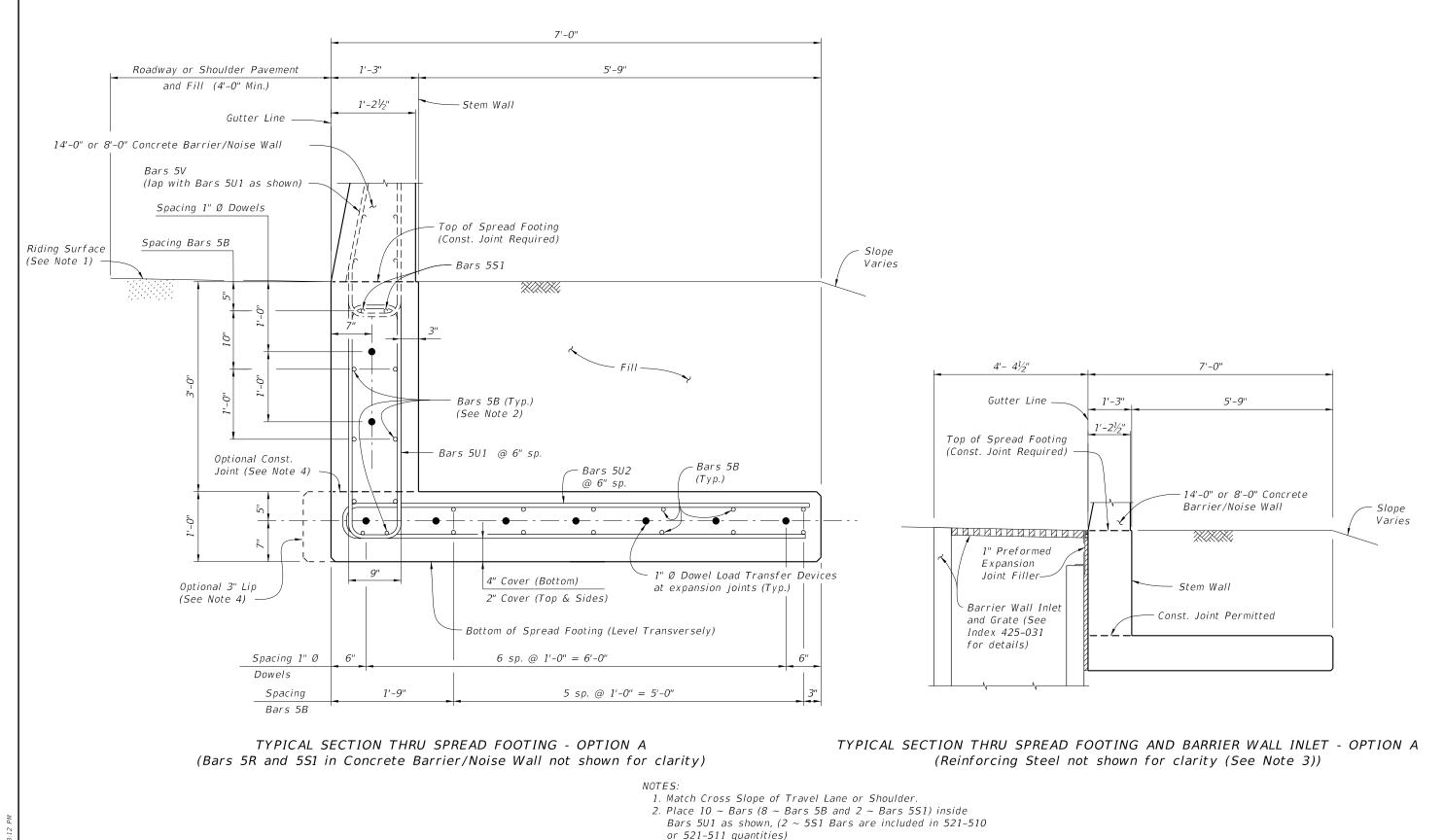
CROSS REFERENCE:

For Detail "A", see Sheet 3. For Section A-A and Estimated Quantities, see Sheet 4.

REVISION 11/01/18

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DESCRIPTION:



3. For Reinforcing Steel spacing, see Typical Section Thru

4. Provide 3" lip when optional construction joint is used.

Spread Footing - Option A this Sheet.

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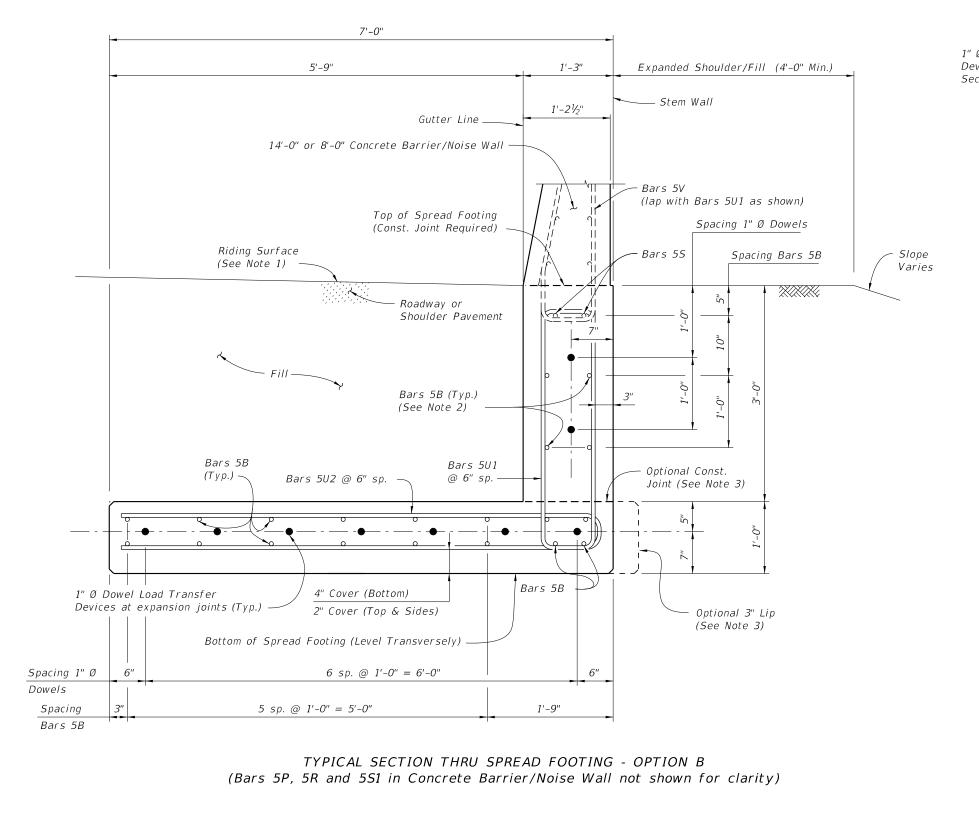
FY 2019-20 STANDARD PLANS

CONCRETE BARRIER/NOISE WALL L-SHAPED SPREAD FOOTING

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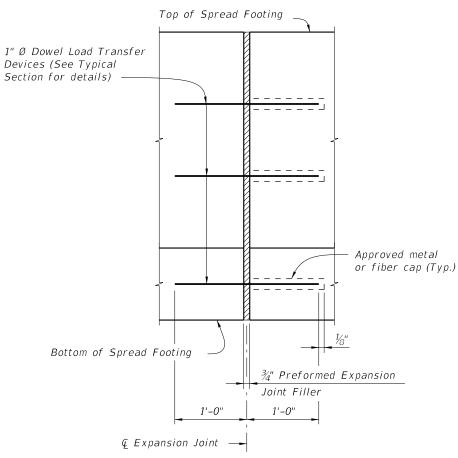
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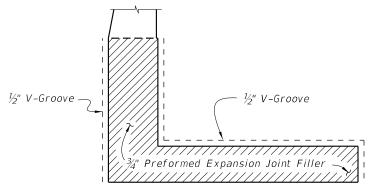
### NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. Place 10  $\sim$  Bars (8  $\sim$  Bars 5B and 2  $\sim$  Bars 5S1) inside Bars 5U1 as shown.
- 3. Provide 3" lip when optional construction joint is used.



# EXPANSION JOINT DETAIL

(Spread Footing expansion joints are required at ¾" open joints in Concrete Barrier/Noise Wall)



DETAIL "A" (Option A Shown, Option B Similar)

(Showing Locations of  $\frac{1}{2}$ " V-Grooves and  $\frac{3}{4}$ " Preformed Expansion Joint Filler)

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LAST REVISION 11/01/17

DESCRIPTION:

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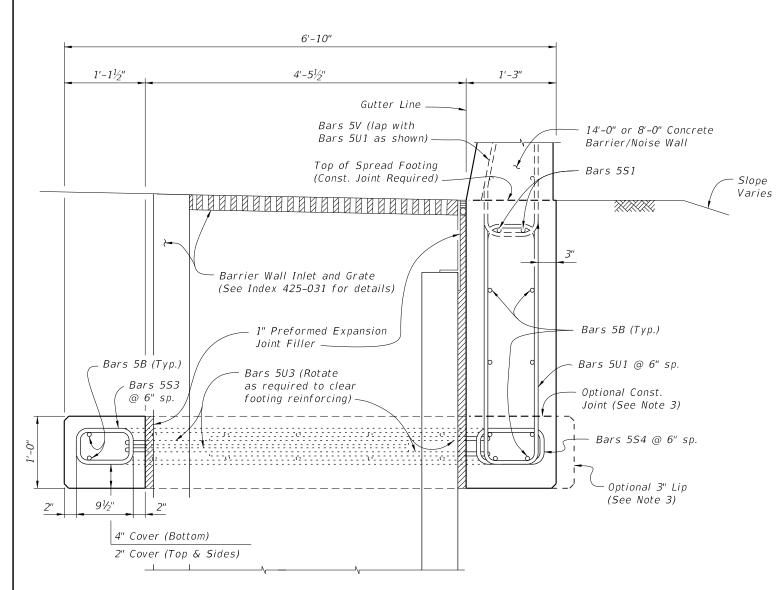
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CONCRETE BARRIER/NOISE WALL L-SHAPED SPREAD FOOTING

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# SECTION A-A TYPICAL SECTION THRU SPREAD FOOTING AND BARRIER WALL INLET - OPTION B (Bars 5P, 5R and 5S1 in Concrete Barrier/Noise Wall not shown for clarity)

- 1. Place 8 ~ Bars 5B and 2 Bars S1 inside Bars 5U1 as shown.
- 2. For Reinforcing Steel spacing, see Typical Section Thru Spread Footing - Option B on Sheet 3.
- 3. Provide 3" lip when optional construction joint is used.

ESTIMATED L-SHAPED SPREAD FOOTING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete (Footing)	CY/FT	0.398	
Reinforcing Steel (Typical) *	LB/FT	68.84	
Additional Reinf. @ Expansion Joint	LB	48.06	

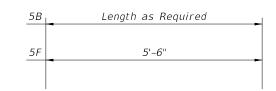
* Bars 5V and 5S1 are included in Index 521-510 or 521-511 quantiles.

CROSS REFERENCE:

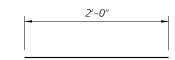
For location of Section A-A, see Sheet 1.

# REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL			
MARK	SIZE	LENGTH	
В	5	AS REQD.	
F	5	5'-6"	
<i>S3</i>	5	3'-7"	
54	5	3'-10"	
U 1	5	9'-2"	
U2	5	13'-10"	
U3	5	12'-10"	
DOWEL	1" Ø Smooth Bar	2'-0"	



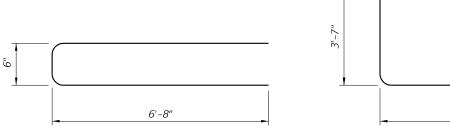
BARS 5B & 5F



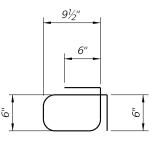
1" Ø DOWEL

5'-8"

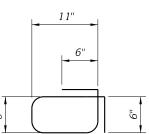
BAR 5U3

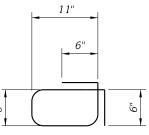


BAR 5U2



**BAR 5S3** 





BAR 5U1

# REINFORCING STEEL NOTES:

BAR 5S4

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-2".
- 4. Lap splices Bars 5T and 5V with 5U1 will be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

**REVISION** 11/01/17

DESCRIPTION:

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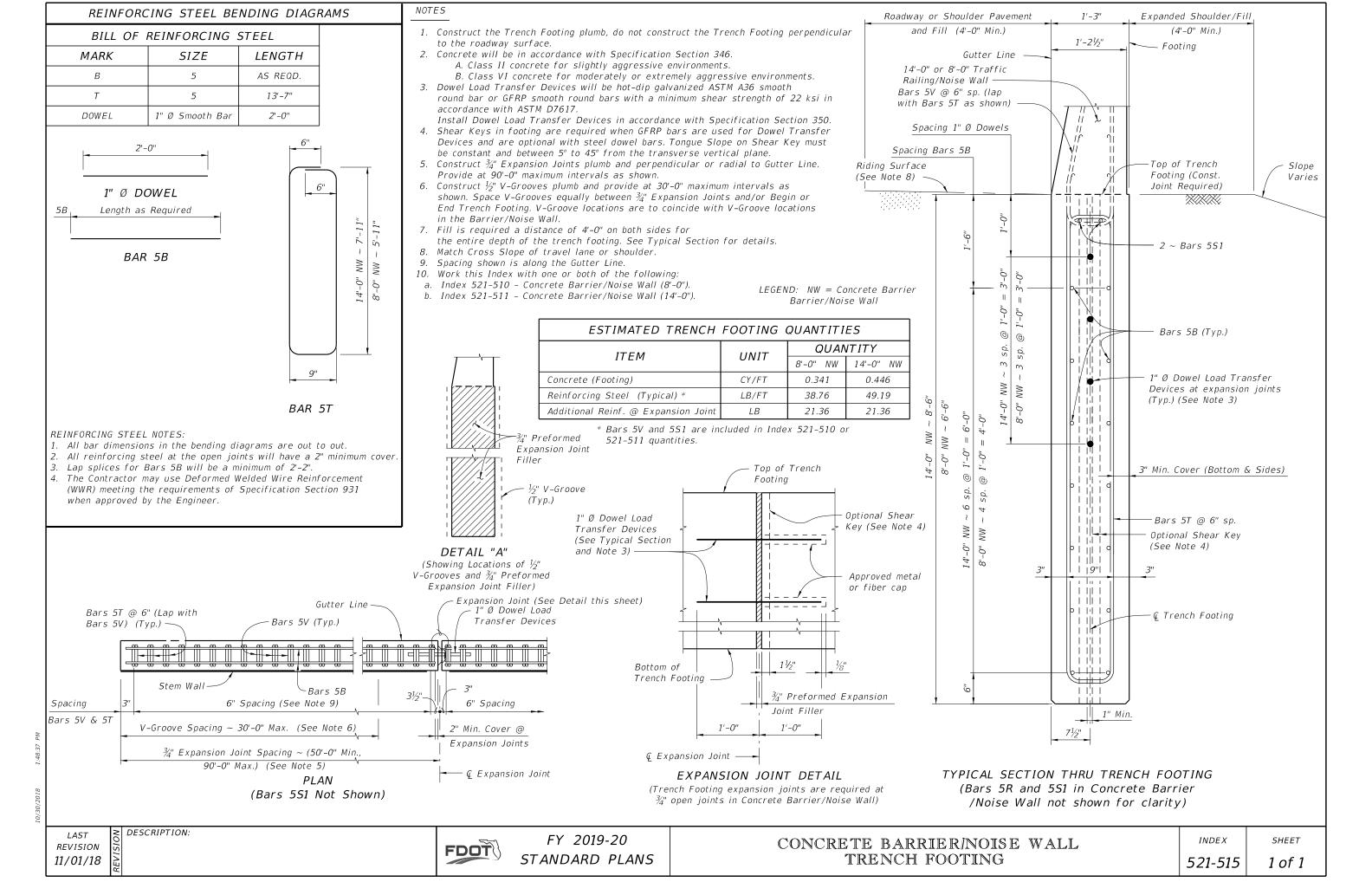
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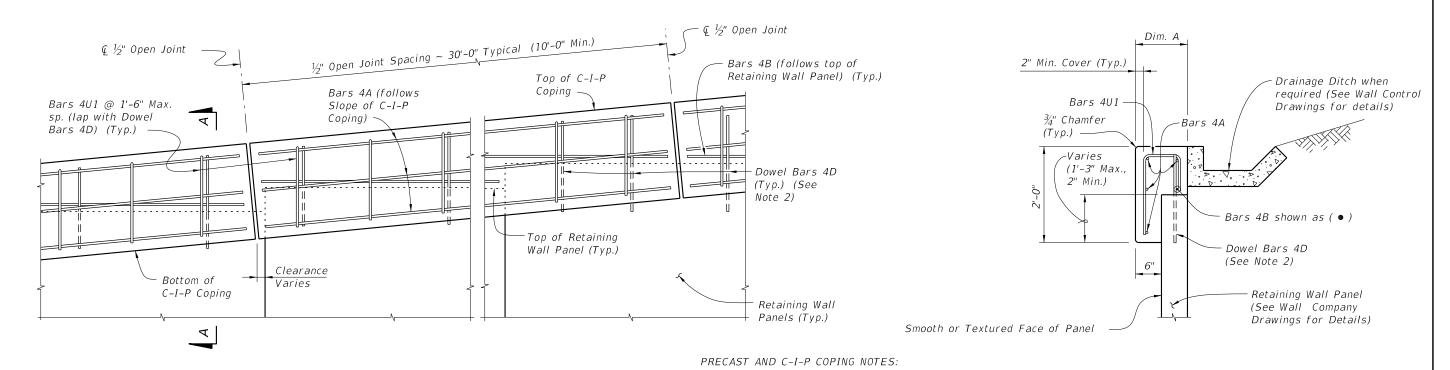
CONCRETE BARRIER/NOISE WALL

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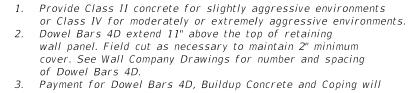
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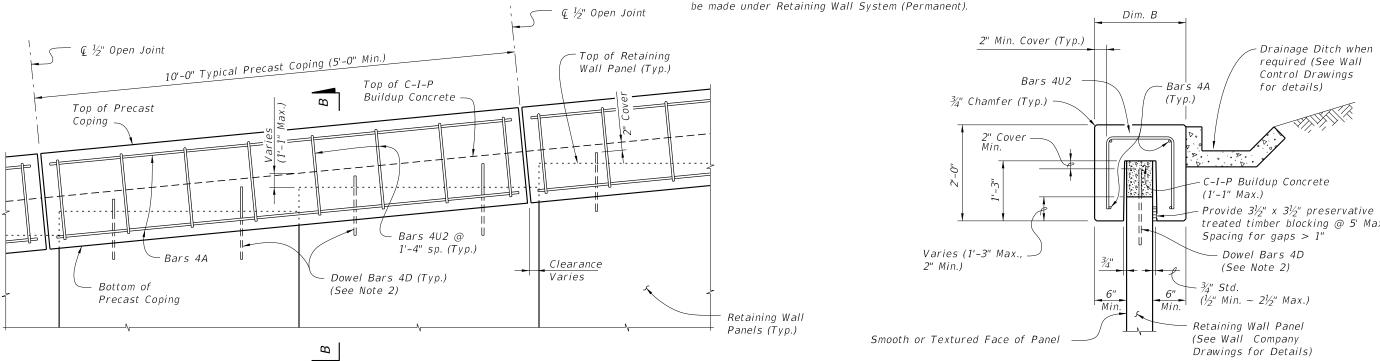
# C-I-P COPING - PARTIAL ELEVATION VIEW



Dim. B 2" Min. Cover (Typ.) Drainage Ditch when required (See Wall Control Drawings Bars 4U2 - Bars 4A for details) (Typ.)¾" Chamfer (Typ.) − 2" Cover -C-I-P Buildup Concrete (1'-1" Max.) Provide  $3\frac{1}{2}$ " x  $3\frac{1}{2}$ " preservative treated timber blocking @ 5' Max. Spacing for gaps > 1" Varies (1'-3" Max., Dowel Bars 4D 2" Min.) (See Note 2) ¾" Std.  $(\frac{1}{2}'')$  Min. ~  $2\frac{1}{2}''$  Max.) Min. Retaining Wall Panel (See Wall Company Drawings for Details)

SECTION A-A

C-I-P COPING



PRECAST COPING - PARTIAL ELEVATION VIEW

SECTION B-B PRECAST COPING

**REVISION** 07/01/14

DESCRIPTION:

FDOT

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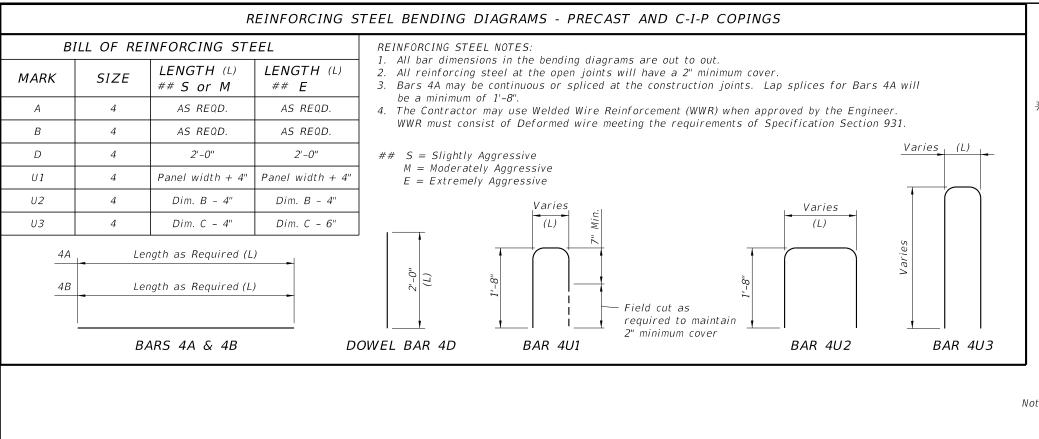
SHEET

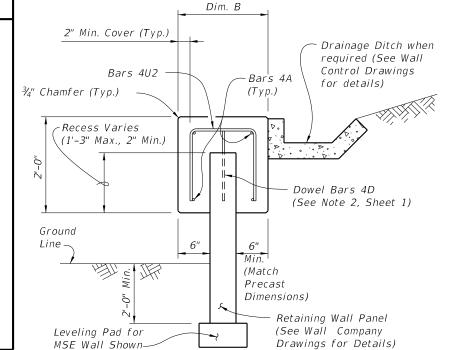
Panel width

+ 6"

Panel width

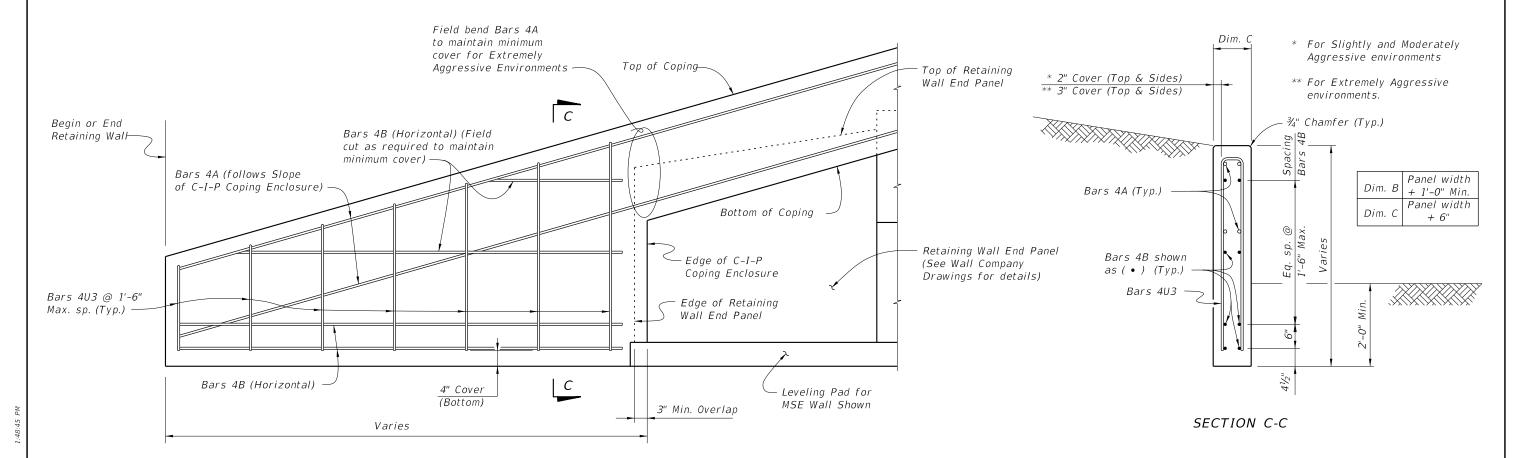
Dim. B + 1'-0" Min.





# C-I-P COPING USED WITH PRECAST COPING

Note: When precast coping units do not fit the entire length of the retaining wall, use this similar C-I-P coping for short portions between precast coping units. This C-I-P coping may also be used for vertical copings.



C-I-P COPING ENCLOSURE DETAIL

REVISION 07/01/13

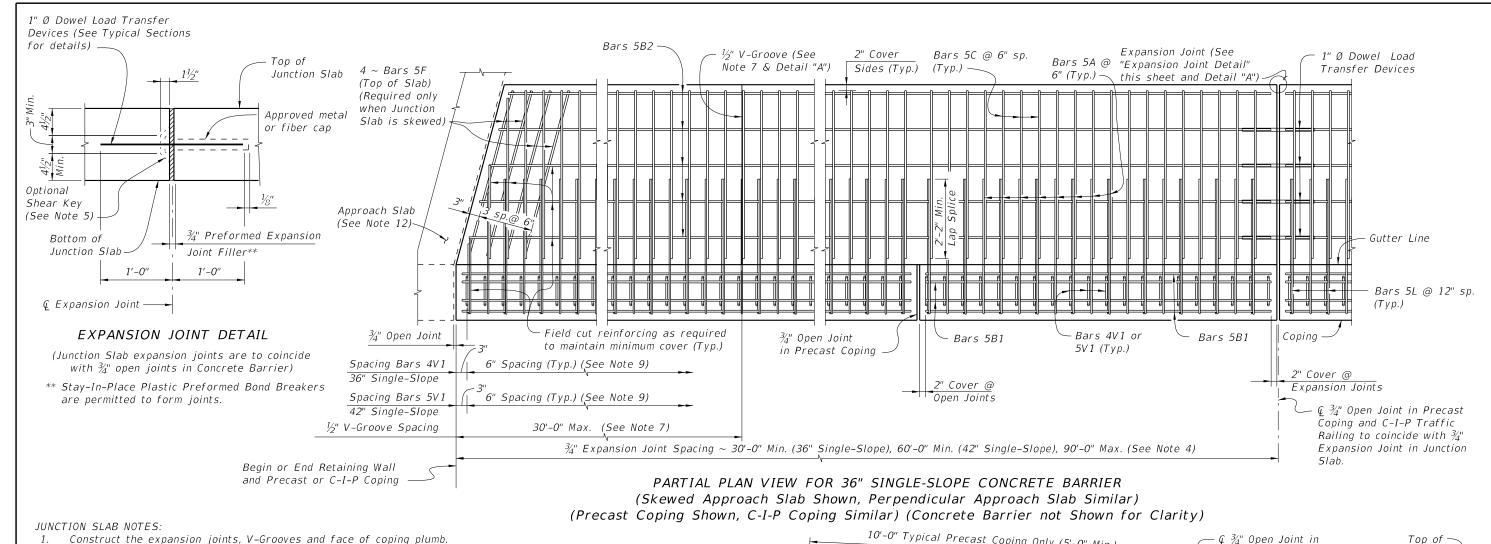
DESCRIPTION:

**FDOT** 

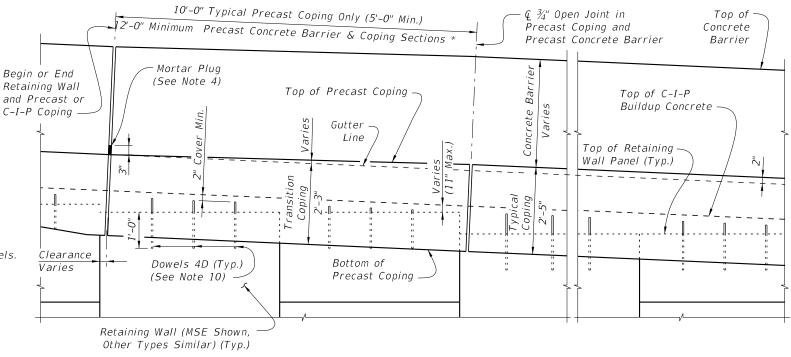
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SHEET



- 2. Provide Class II concrete for slightly aggressive environments or Class IV for moderate or extremely aggressive environments.
- 3. Dowel Load Transfer Devices will be hot-dip galvanized ASTM A 36 smooth round bar, or GFRP smooth round bars with a minimum shear strength of 22 ksi in accordance with ASTM D7617. Install Dowel Load Transfer Devices in accordance with Specification Section 350.
- Construct ¾" Expansion Joints in junction slabs and C-I-P copings plumb and perpendicular or radial to the Gutter Line. Provide at 90'-0" maximum intervals as shown. Provide 3"x3" Mortar plugs in open joints at the base of Concrete Barriers to contain runoff.
- 5. Shear Keys in Junction Slab are required when GFRP bars are used for Dowel Transfer Devices and are optional with steel dowel bars. Tongue Slope on Shear Key must be constant and between 5° to 45° from horizontal.
- 6. Provide and install Preformed Expansion Joint Filler in accordance with Specification Section 932.
- 7. Construct ½" V-Grooves in junction slabs and C-I-P copings at 30'-0" maximum intervals as shown. Space V-Grooves equally between ¾" Expansion Joints and/or Begin or End Junction Slab. V-Groove locations are to coincide with V-Groove locations in the Concrete Barrier.
- 8. Shoulder or Roadway Pavement is required on top of the junction slab for its entire length on the traffic side of the Concrete Barrier. See Typical Sections on Sheets 2 and 3 for details.
- 9. Spacing shown is along the Gutter Line.
- 10. For Precast Coping only, provide Dowel Bars 4D embedded 1'-0" and extended 9" above the top of MSE wall panels. Field cut as necessary to maintain 2" minimum cover to the top of the buildup concrete. See Wall Company Drawings for number and spacing of Dowel Bars 4D.
- 11. The following Indexes contain details of the intersection of the retaining wall at approach slabs: Index 400-090 - Approach Slabs (Flexible Pavement Approaches) Index 400-091 - Approach Slabs (Rigid Pavement Approaches)



PARTIAL ELEVATION VIEW

(Precast Coping and Junction Slab Reinforcing not Shown for Clarity)

(Precast Coping Shown, C-I-P Coping Similar)

* C-I-P End Section must  $be \ge 12'-0"$ .

CROSS REFERENCE: For Detail "A", see Sheet 2.

IAST ≥ DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

CONCRETE BARRIER/JUNCTION SLAB
- WALL COPING

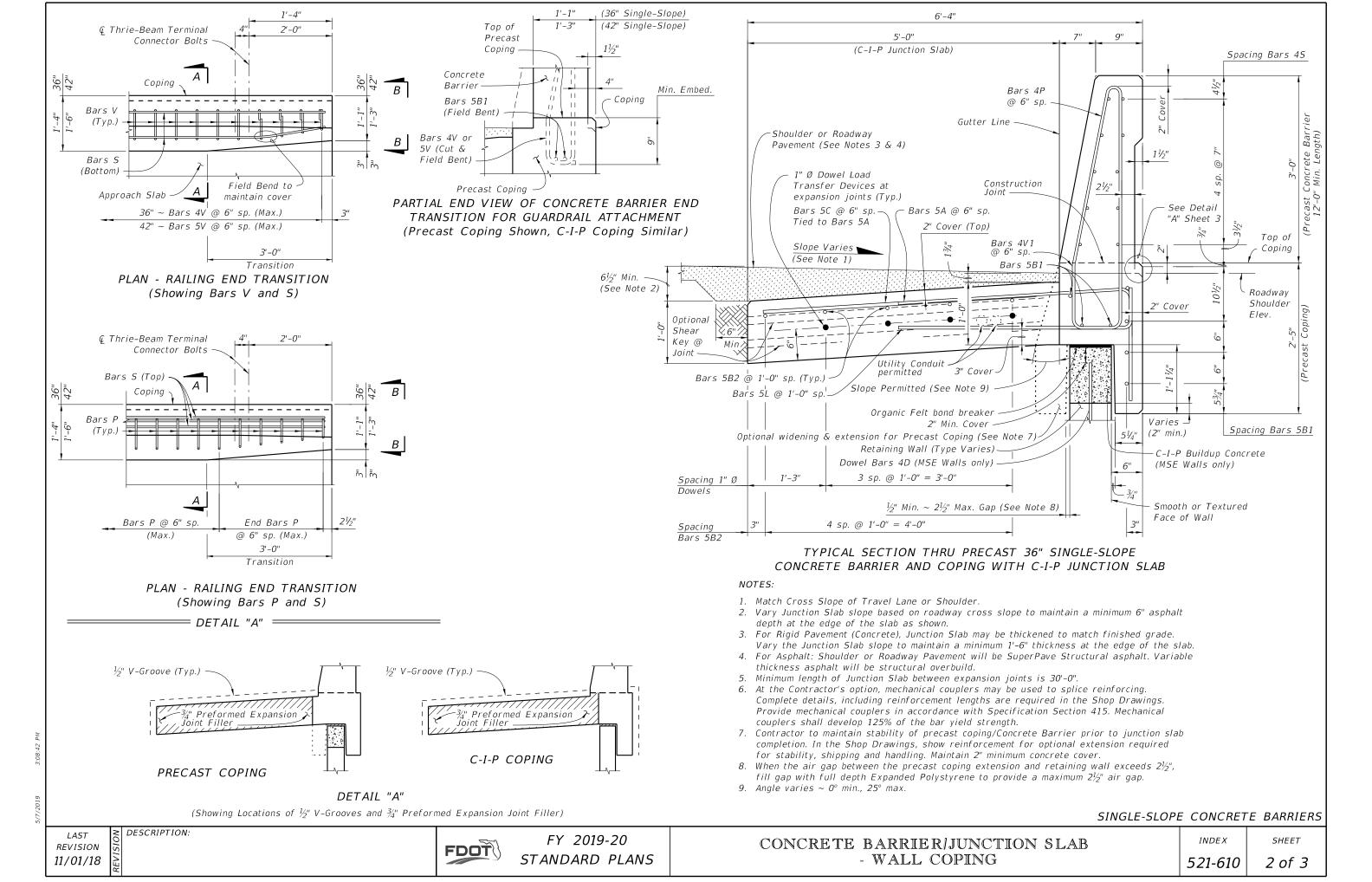
INDEX SHEET

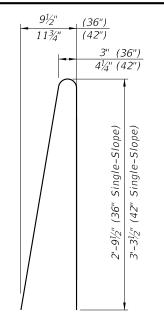
SINGLE-SLOPE CONCRETE BARRIERS

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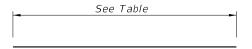
REVISION

11/01/17



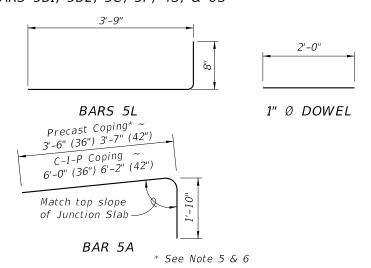


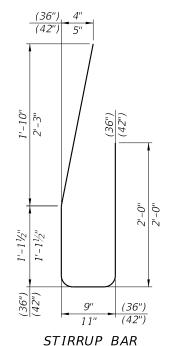
STIRRUP BAR 4P (36") 5P (42")



Dowel

BARS 5B1, 5B2, 5C, 5F, 4S, & 6S



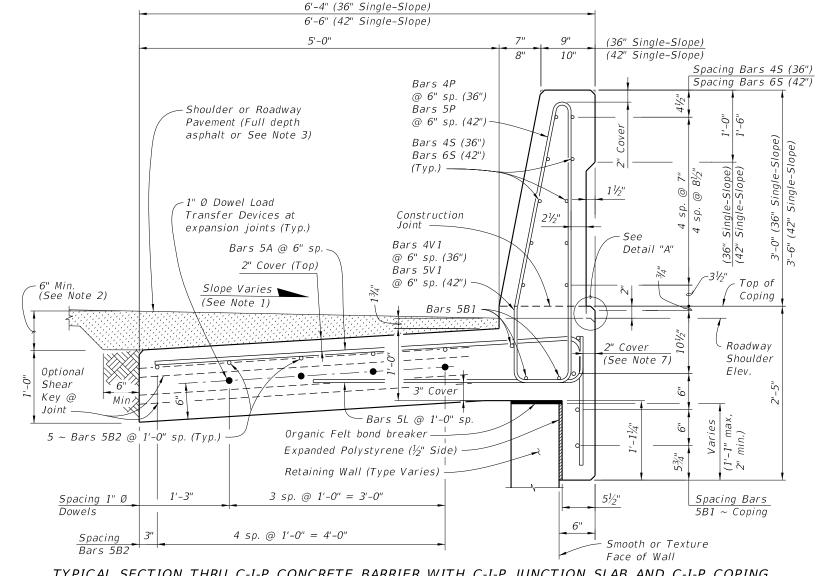


4V1 (36") 5V1 (42")

#### REINFORCING STEEL NOTES:

DESCRIPTION:

- 1. All bar dimensions in the bending diagrams are out to out.
- All reinforcing steel at expansion and open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B & 5S will be a minimum of 2'-2".
- 4. For Precast Copings only, lap splice Bars 5A with Bars 5C. Lap splices will be a minimum of 2'-2".
- 5. The Contractor may use either full length Bars 5A or lap splice with Bars 5C at Bars 5A for C-I-P Copings.
- 6. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 1'-2\frac{1}{2}'' (36" Single-Slope) or  $1'-4\frac{1}{2}$ " (42" Single-Slope).
- 7. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 4'-8".
- 8. When approved by the Engineer, the Contractor may use deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931.
- 9. Contractor may use a single #5 stirrup in lieu of two bars for 4P and 4V1.

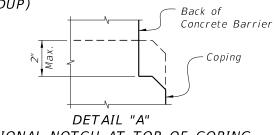


TYPICAL SECTION THRU C-I-P CONCRETE BARRIER WITH C-I-P JUNCTION SLAB AND C-I-P COPING (PRECAST COPING SIMILAR WITH C-I-P BUILDUP) NOTES:

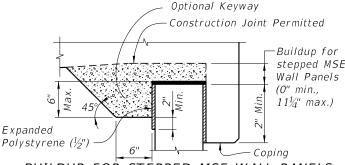
- 1. Match Cross Slope of Travel Lane or Shoulder
- 2. Vary the Junction Slab slope based on the roadway cross slope to maintain a minimum 6" asphalt depth at the edge of the slab.
- 3. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finish grade. Vary the Junction Slab slope to maintain a minimum 1'-6" thickness at the inside edge of the slab.
- 4. Minimum length of Junction Slab between expansion joints is 30'-0" for 36" Single-Slope or 60'-0" for 42" Single-Slope.
- Contractor to maintain stability of precast coping prior to junction slab completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 2" minimum concrete cover.
- 6. If slip forming is used, submit shop drawings for approval showing  $2\frac{1}{2}$  side cover with the Typical Section dimensions adjusted.

ESTIMATED QUANTITIES FOR C-I-P				
ITEM	UNIT	QUANTITY (36")	QUANTITY (42")	
Concrete	CY/LF	0.376	0.420	
Reinforcing Steel (Typical) (excludes Bars 5C & 5F)	LB/LF	62.45	82.17	
Additional Reinf. @ Expansion Joint (Steel Dowels)	LB	21.36	21.36	

(The above concrete quantities are based on a max. superelevation of 6.25%)



OPTIONAL NOTCH AT TOP OF COPING



BUILDUP FOR STEPPED MSE WALL PANELS AND C-I-P COPING

SINGLE-SLOPE CONCRETE BARRIERS

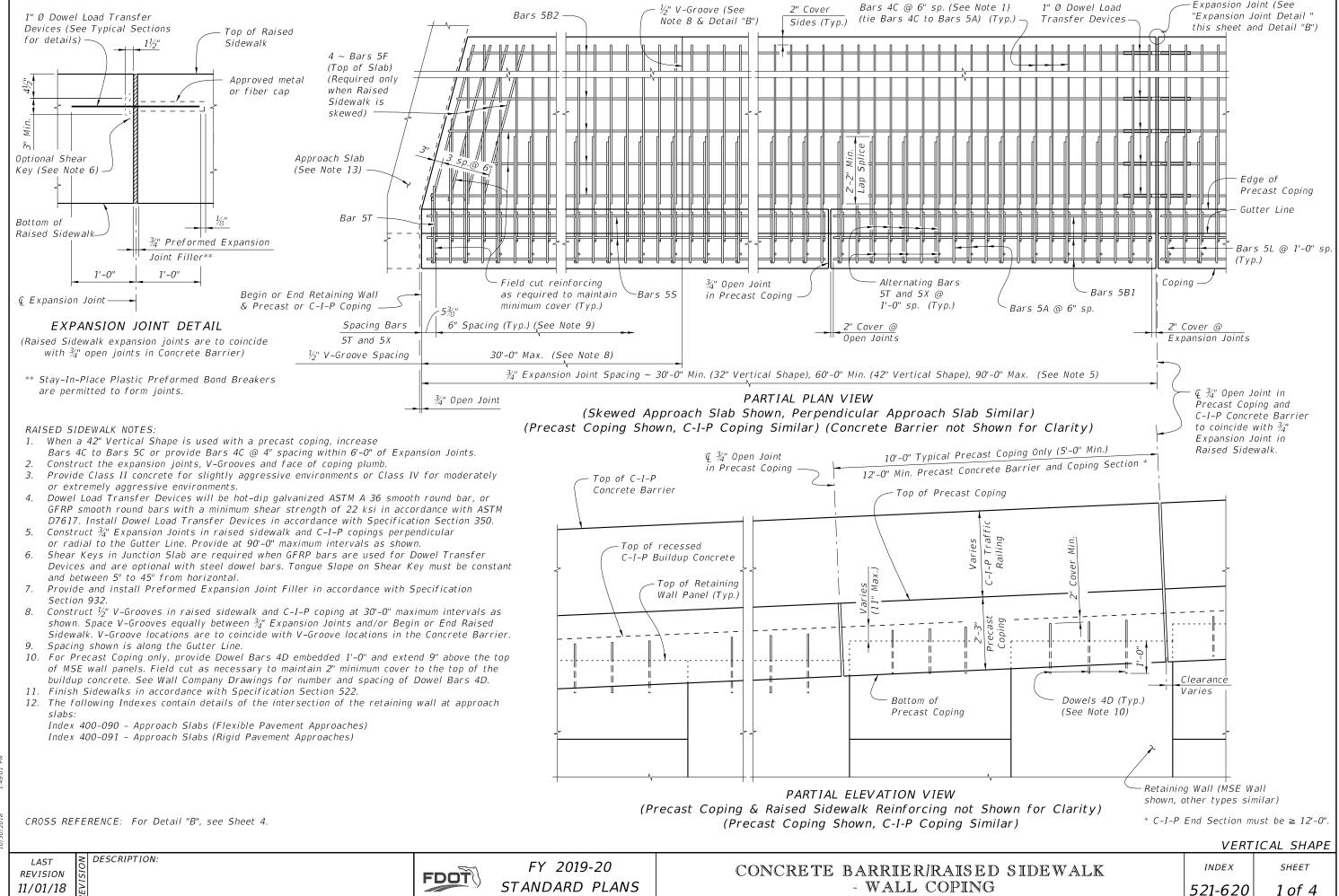
REVISION 11/01/17



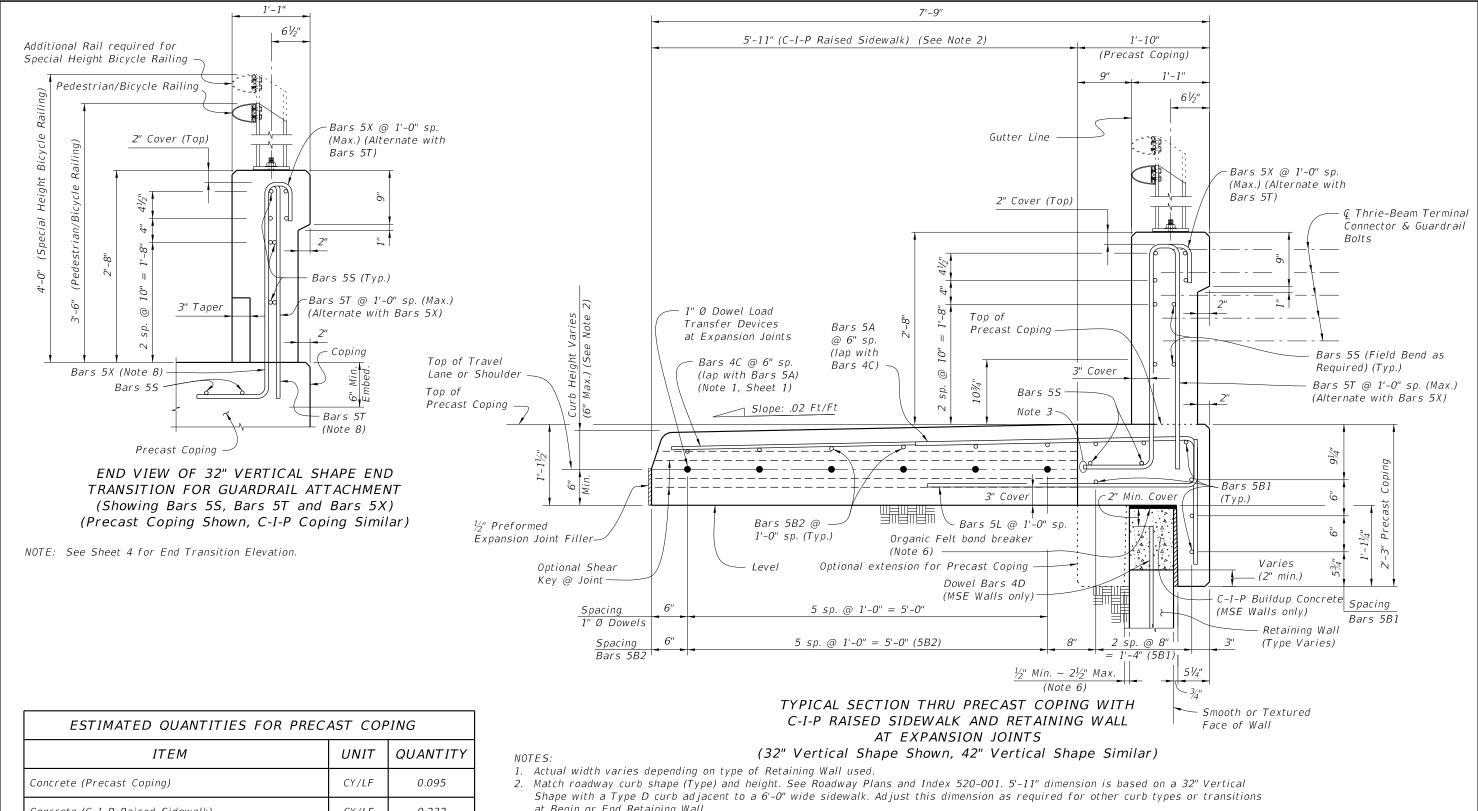
FY 2019-20 STANDARD PLANS

- WALL COPING

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0100/00/01



Concrete (C-I-P Raised Sidewalk) CY/LF 0.232 Reinforcing Steel (Precast Coping) excluding LB/LF 23.90 Bars 5T, 5X and 5S (Typ.) Reinforcing Steel (C-I-P Raised Sidewalk) (Typ.) LB/LF 13.50 Additional Reinf. @ Expansion Joints (Steel Dowels) 32.04

The above concrete quantities are based on a Type D Concrete Curb (See Note 2).

DESCRIPTION:

- at Begin or End Retaining Wall.
- 3. Trim end of Bars 5T and 5X to clear construction joint for 42" Vertical Shape.
- 4. At the Contractor's option, mechanical couplers may be used to splice reinforcing. Complete details, including reinforcement lengths are required in the Shop Drawings. Mechanical couplers shall develop 125% of the bar yield strength.
- 5. Contractor to maintain stability of precast coping prior to junction slab completion.
- 6. When the air gap between the precast coping extension and retaining wall exceeds  $2\frac{1}{2}$ ", fill gap with full depth Expanded Polystyrene to provide a maximum  $2\frac{1}{2}$ " air gap.
- 7. For Bullet Railings, see Index 515-021 and 515-022.
- 8. Begin placing Railing Bars 5T and 5X at the railing end and proceed toward Retaining Wall to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in End Transition.

32" VERTICAL SHAPE

**REVISION** 11/01/18

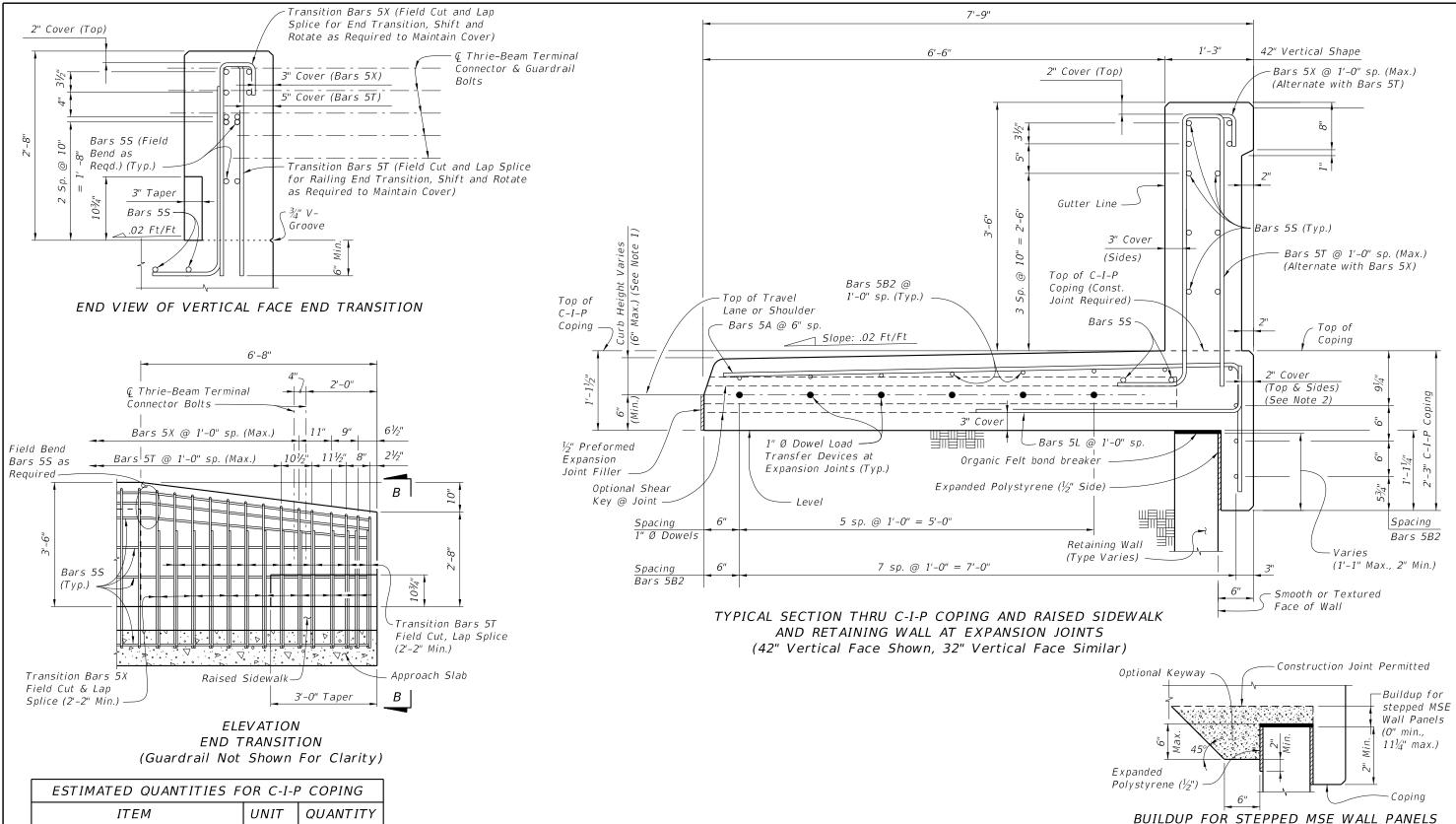
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FY 2019-20 STANDARD PLANS

CONCRETE BARRIER/RAISED SIDEWALK - WALL COPING

INDEX SHEET

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ITEM UNIT QUANTITY Concrete CY/LF 0.326 Reinforcing Steel (Typical) excluding LB/LF 35.38 Bars 5T, 5X and 5S (Typ.)

The above concrete quantities are based on a Type D Concrete Curb on a level Retaining Wall (See Note 1).

Additional Reinf. @ Expansion Joints

NOTES:

- 1. Match roadway curb shape (Type) and height. See Roadway Plans and Index 520-001. 6'-6" dimension is based on a 42" Vertical Shape with a Type D curb adjacent to a 6'-0" wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.
- 2. If slip forming is used, submit shop drawings for approval showing 3" side cover with the Typical Section dimensions adjusted.
- 3. Begin placing Railing Bars 5T and 5X at the railing end and proceed toward Retaining Wall to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Retaining Wall. Cut, shift and rotate Bars 5T and 5X as required to maintain cover in End Transition. 42" VERTICAL SHAPE

DESCRIPTION: 11/01/18

32.04

FDOT

FY 2019-20 STANDARD PLANS

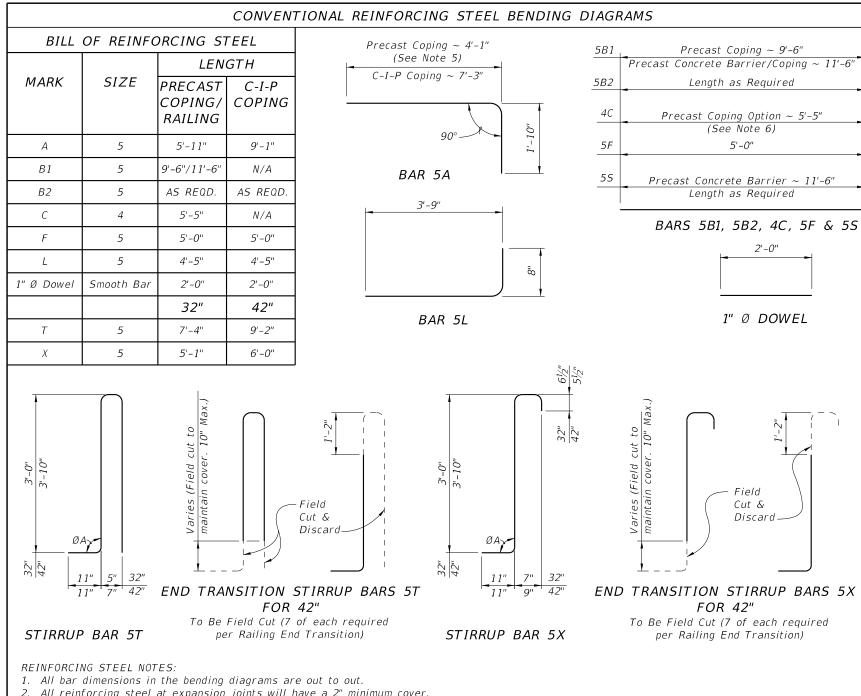
CONCRETE BARRIER/RAISED SIDEWALK - WALL COPING

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AND C-I-P COPING

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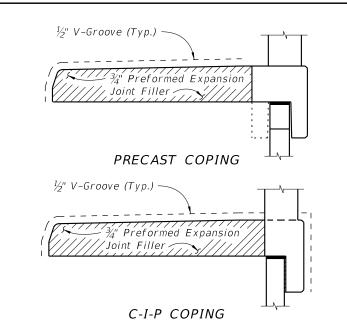
(Steel Dowels)



- 2. All reinforcing steel at expansion joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B and 5S will be a minimum of 2'-2".
- 4. Lap splice Bars 5A with Bars 4C will be a minimum of 2'-2".
- 5. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 1'-8".
- 6. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 5'-8", and reinforcing size must be increased to #5 bars (Bars 5C).
- 7. The Contractor may use deformed WWR when approved by the Engineer. WWR must meet the requirements of Specification Section 931.

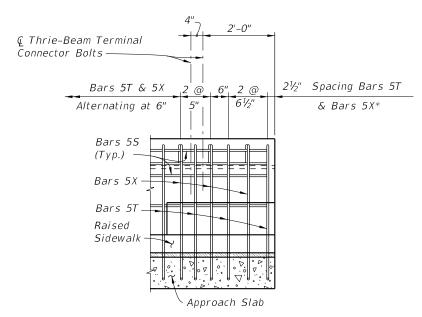
* See Sheet 3 Note 3.

DESCRIPTION:



### DETAIL "B"

(Showing Locations of ½" V-Grooves and 3/4" Preformed Expansion Joint Filler)



END TRANSITION ELEVATION FOR 32" VERTICAL SHAPE (Guardrail Not Shown For Clarity)

ESTIMATED CONCRETE BARRIER QUANTITIES			
	QUANTITY		
ITEM	UNIT	<i>32</i> "	42"
Concrete	CY/LF	0.095	0.145
Reinforcing Steel	LB/LF	23.38	28.33

VERTICAL SHAPE

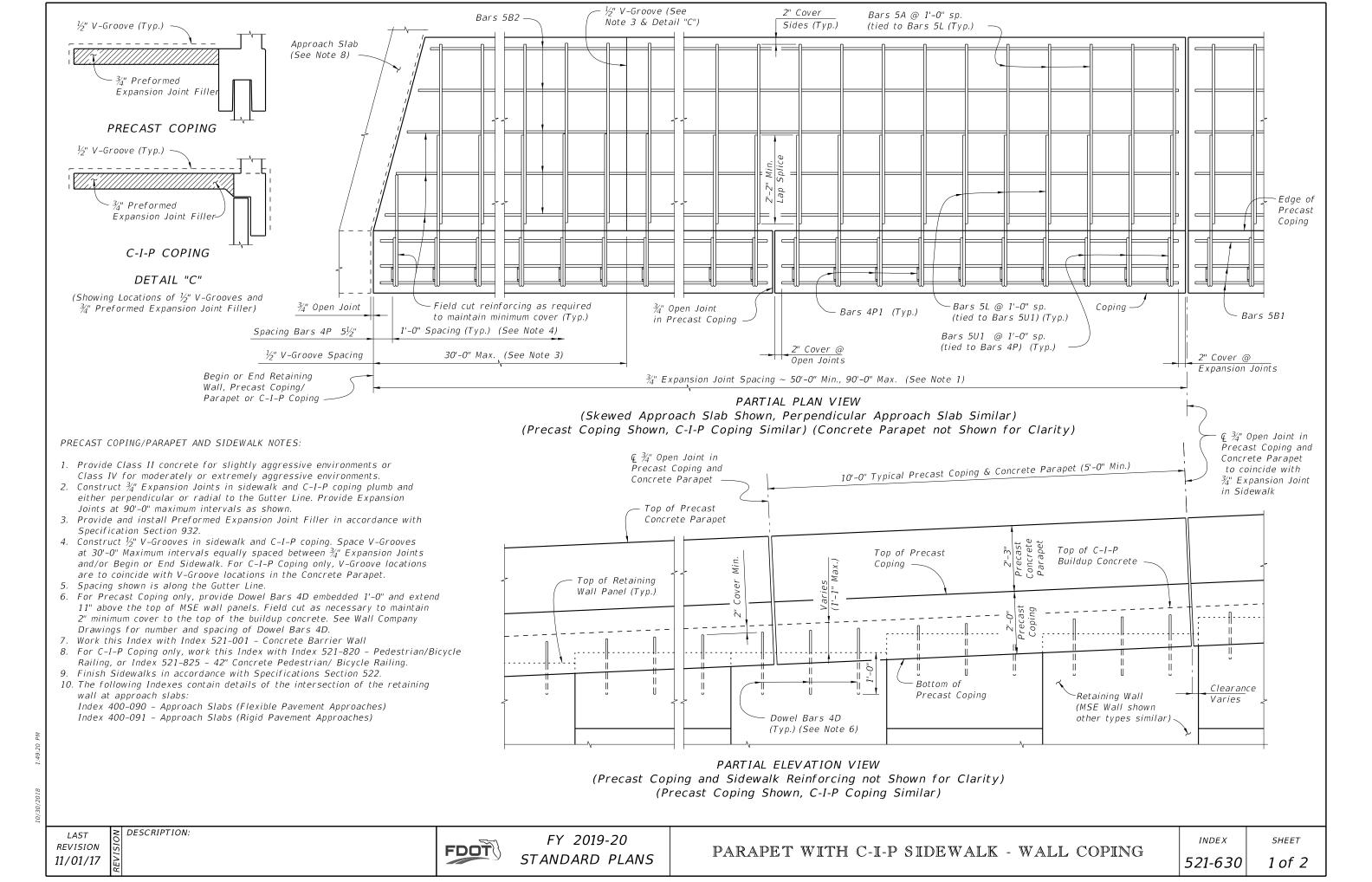
**REVISION** 11/01/18

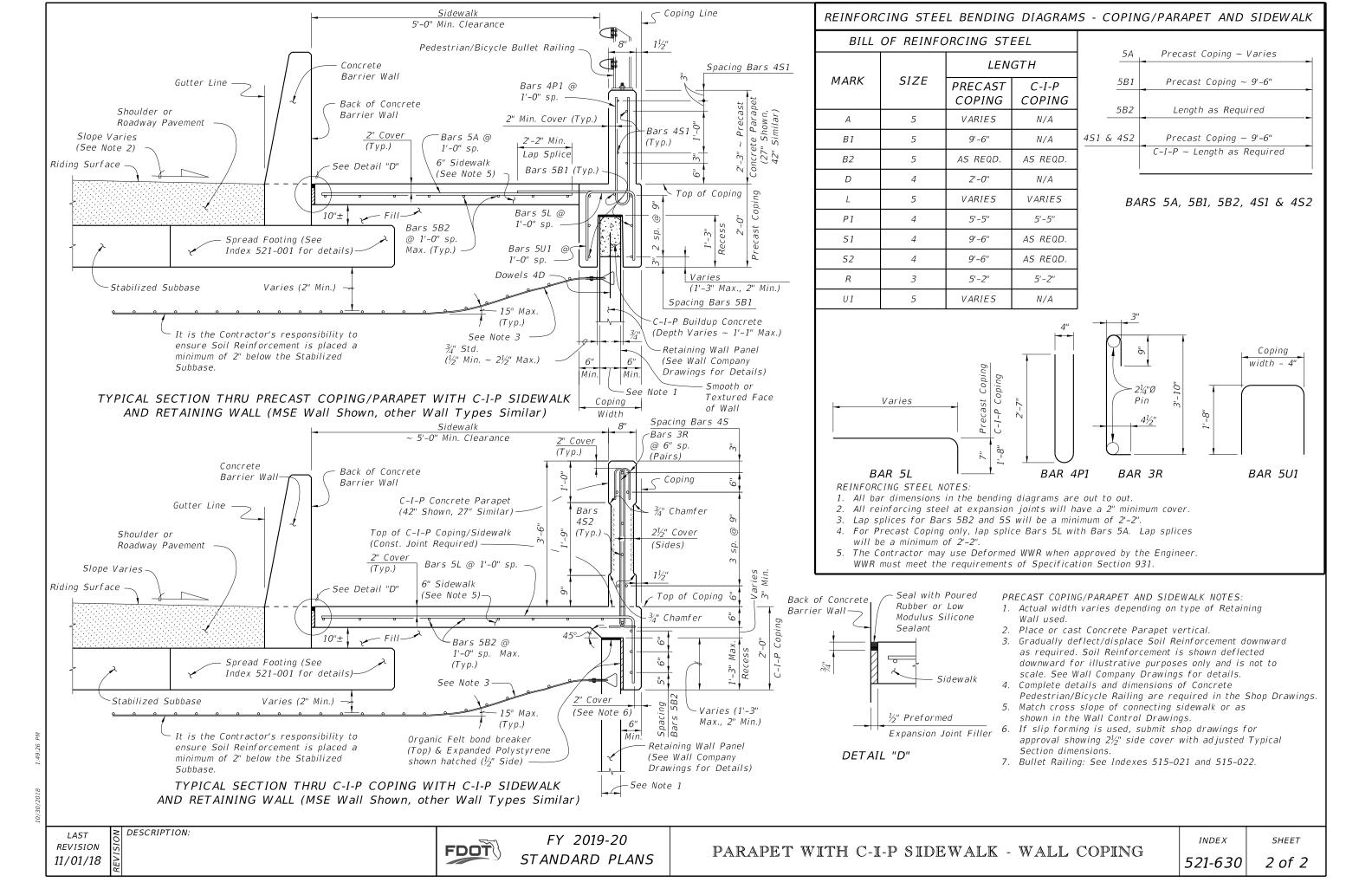


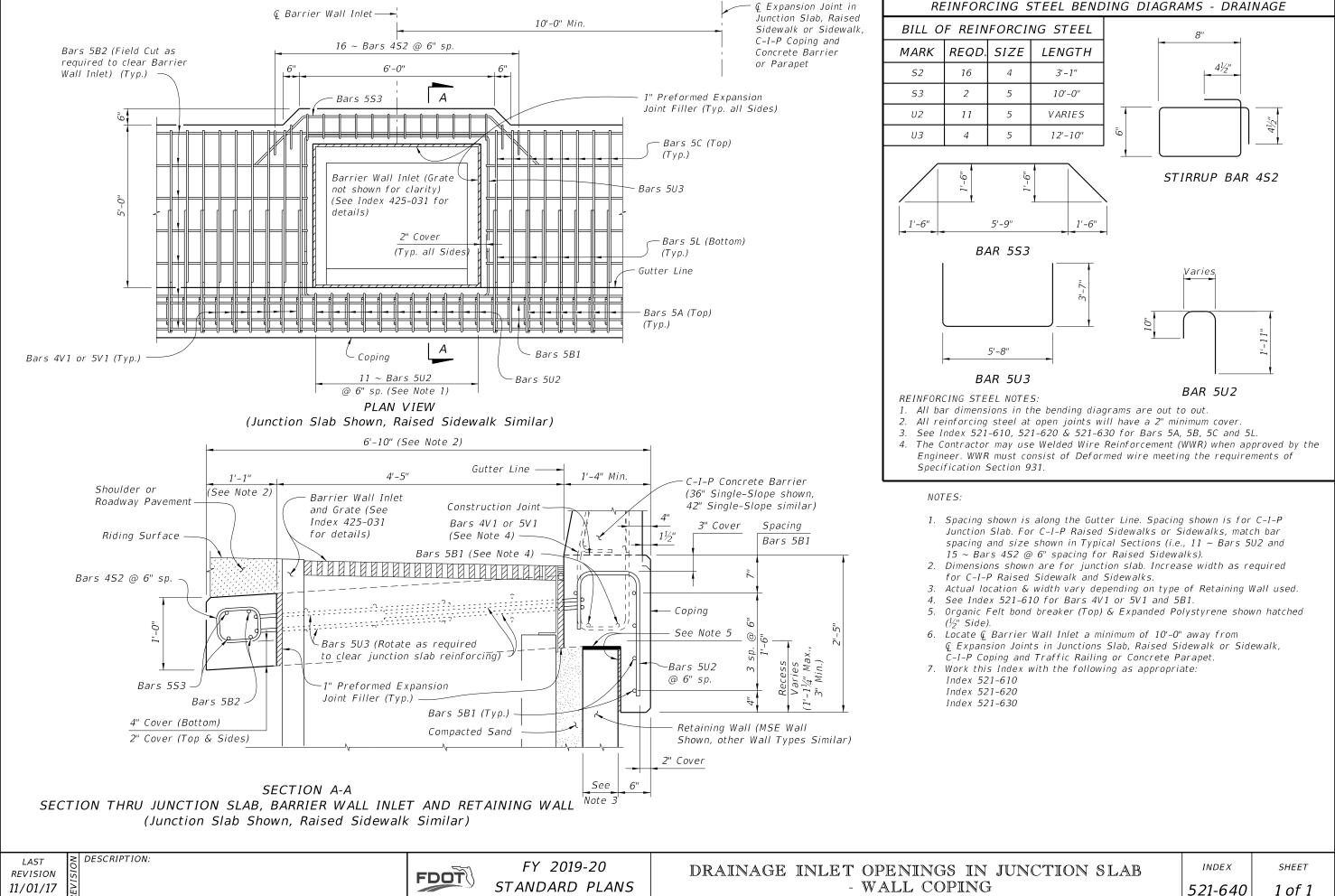
FY 2019-20 STANDARD PLANS CONCRETE BARRIER/RAISED SIDEWALK

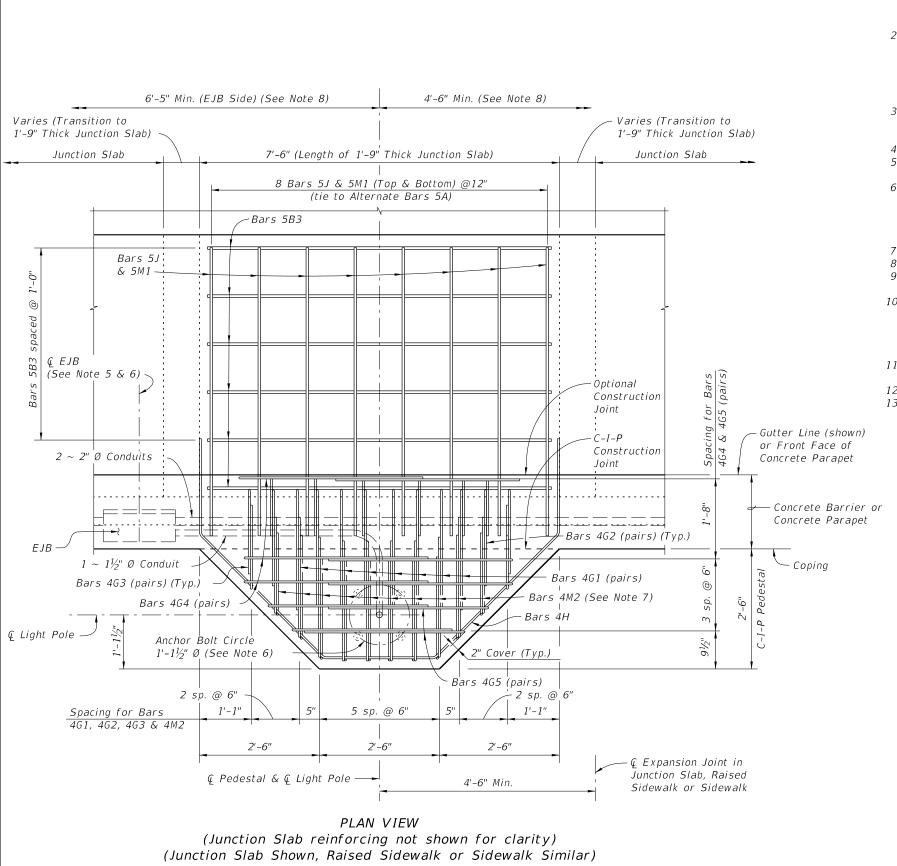
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LIGHT POLE PEDESTAL NOTES:

1. ANCHOR BOLTS:

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole configurations shown on Index 715-040 with top of pedestal 75' or less above ground or MLW.

Anchor Bolt Diameter: See Table 1

2. MATERIALS:

Anchor Bolts: ASTM F1554 Grade 55.

Nuts: ASTM A563 Grade A, Heavy-Hex.

Washers: ASTM F436 Type 1.

Anchor Plate: ASTM A709 (Grade 36) or ASTM A36.

Coating: Galvanize all Nuts, Bolts Washers, and plates in accordance with ASTM F2329.

- 3. The Contractor is responsible for ensuring the anchor bolt design is compatible with the light pole base plate. Modifications to the anchor bolt design shown must be signed and sealed by the Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.
- 4. Install Anchor Bolts plumb.
- 5. For conduit, EJB and expansion/deflection fitting details, see Utility Conduit Detail Drawings.
- 6. The cost of anchor bolts, nuts, washers and anchor plates will be included in the Bid Price for Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Concrete Barrier or Concrete Parapet that the pedestal is behind.
- 7. Field Cut Bars 4M2 as required to maintain clearance.
- 8. Slip Forming Method of construction requires the Engineer's approval within the limits shown.
- Reinforcing shown for light pole pedestals is in addition to typical reinforcing for Junction Slabs and Raised Sidewalks.
- 10. Work this Index with the following as appropriate:

Index 521-512 Index 521-610

Index 521-620

Index 521-630

- 11. Pedestal may be precast in one section with Coping. Minimum Precast Coping section length is 10 ft. or 12 ft for combination Precast Concrete Barrier and Coping section.
- 12. For Estimated Quantities, see Sheet 3.
- 13. Unless otherwise noted, Concrete Barrier (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other Concrete Barriers or pedestrian/bicycle railings are similar.

TABLE 1 DESIGN LIMITATION FOR ANCHOR BOLTS (1" Dia.)				
Wind Speed	Arm Length	BASE OF	POLE I	HEIGHT*
(MPH)	(FT)	40 ft.	45 ft.	50 ft.
120	ALL	75	75	75
140	ALL	75	75	75
160	8 & 10	75	75	45**
160	12 & 15	75	75	25*

- * Above Natural Ground
- ** Use  $1\frac{1}{4}$ " Ø Anchor bolts for wall heights greater than the height shown and less than 75'.

DESCRIPTION: LAST **REVISION** 11/01/17

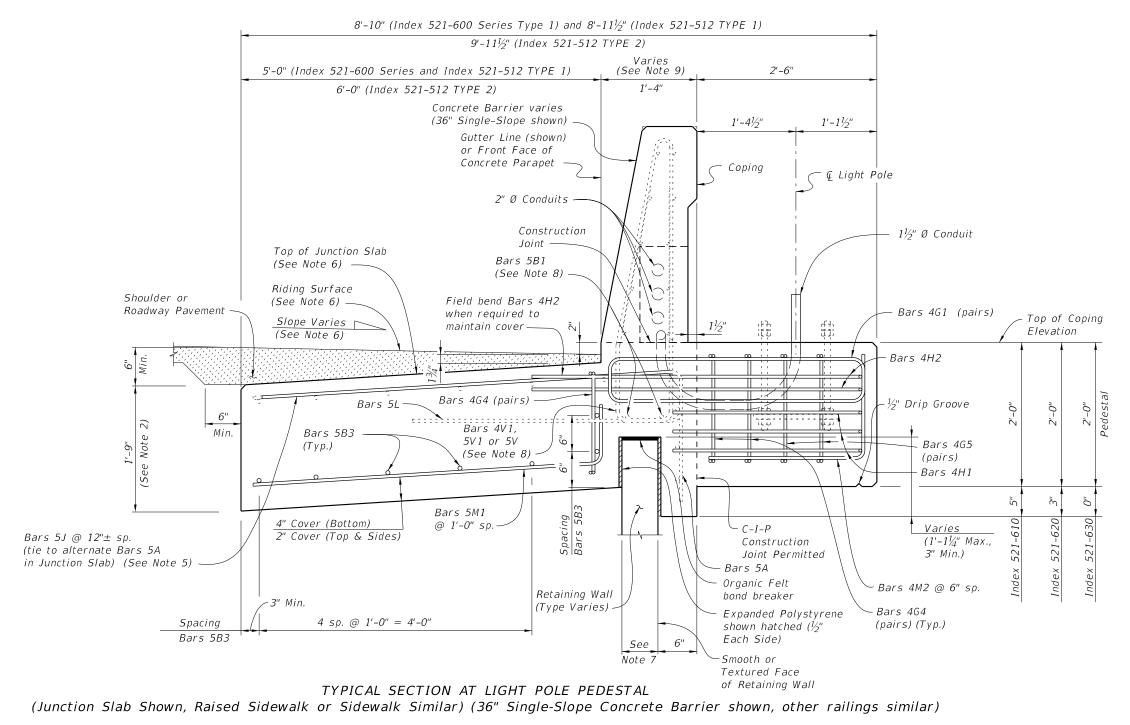


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LIGHT POLE PEDESTAL - WALL COPING *521-650* 

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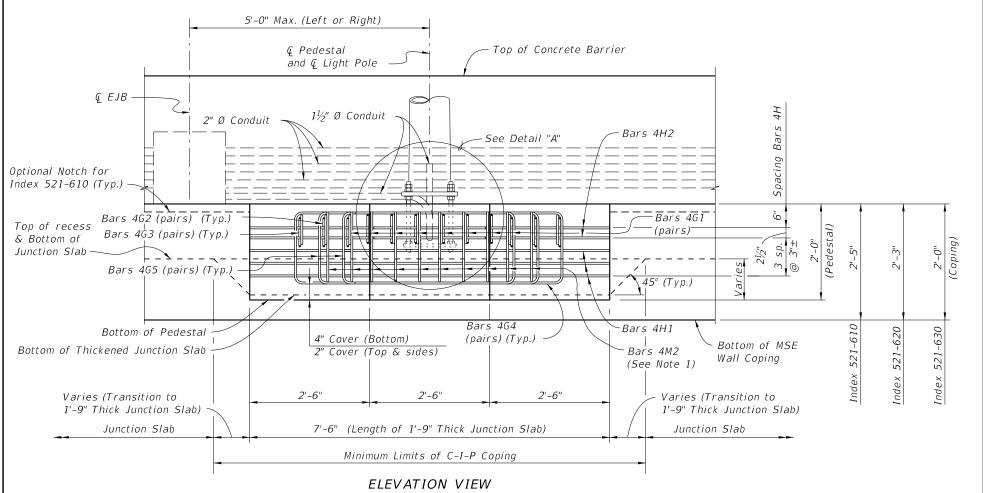


## NOTES:

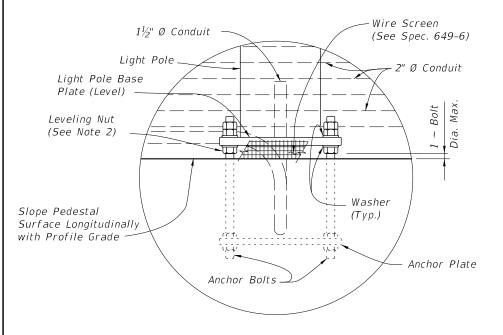
- 1. Provide Concrete Class to match adjacent coping.
- 2. For junction slabs, increase the 1'-0" depth dimension to 1'-9".
- 3. For Parapet with sidewalk see Index 521-630, but increase 6" sidewalk depth to 1'-6". For raised sidewalk see Index 521-620.
- 4. The minimum length of the Junction Slabs, raised sidewalks and sidewalks is 30'-0", measured along the Gutter Line.
- Bars 4J are only required when pedestals are behind a Concrete Barrier or Concrete Barrier/ Noise Wall.
- 6. Top of junction slab may be thickened to match finished grade of concrete pavement or shoulder, or top of sidewalk or raised sidewalk (See Notes 3 & 4).
- 7. Actual width varies depending on type of retaining wall used.
- 8. See Index 521-610 for Bars 4V1, 5V1 and 5B, or Index 521-512 for Bars 5V and 5B1.
- 9. Work with Index 521-512 (Concrete Barrier/ Noise Wall), Index 521-610 (Single-Slope), Index 521-620 (Vertical Shape), and Index 521-630 (Concrete Parapet).

10/30/2018

DESCRIPTION:



(Junction Slab Reinforcing & Bars 4J not Shown for Clarity) (Junction Slab Shown, Raised Sidewalk or Sidewalk Similar)



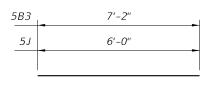
- 1. Field Cut Bars 4M2 as required to maintain minimum cover.
- 2. Maximum clearance between leveling nut and top of pedestal will not exceed anchor bolt diameter.

ESTIMATED QUANTITIES				
ITEM UNIT QUANTITY				
Concrete (Pedestal)	CY	0.926		
Concrete (Thickened Junction Slab)	CY	1.222		
Reinforcing Steel	LB	334.09		

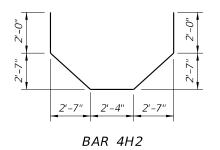
(The quantities above are for one C-I-P Light Pole Pedestal. The concrete quantity for the thickened junction slab is based on a 5'-0" length, 9" increase in thickness and a 5" wide retaining wall panel. Adjust thickened concrete quantity as required.

# REINFORCING STEEL BENDING DIAGRAMS - LIGHT POLE PEDESTAL

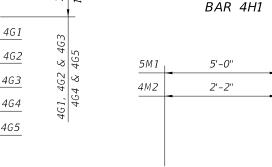
BILL OF REINFORCING STEEL				
MARK	SIZE	NO. REQD.	LENGTH	
В3	5	7	7'-2"	
G 1	4	16	5'-8"	
G2	4	4	4'-8"	
G3	4	4	4'-2"	
G4	4	6	8'-10"	
G5	4	4	7'-4"	
H1	4	3	9'-8"	
H2	4	2	13'-8"	
J	5	8	6'-0"	
M 1	5	8	5'-10"	
M2	4	10	3'-8"	



BARS 5B3 & 5J



2'-4"



BARS 4G1, 4G2, 4G3, 4G4 & 4G5

2'-6" 2'-0"

1'-9"

3'-8"

2'-11"

BAR 5M1 & 4M2

### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. Lap splices for Bars 4G1, 4G2, 4G3, 4G4 & 4G5 will be a minimum of 1'-4".
- 3. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

**REVISION** 11/01/17

DESCRIPTION:

DETAIL "A"



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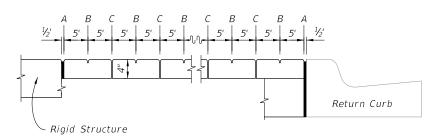
SHEET 3 of 3

5M1 4M2

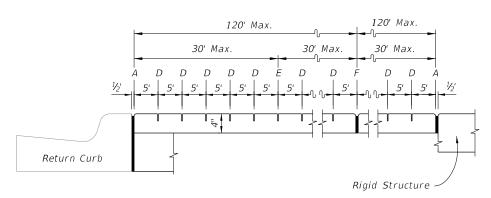
10"

## GENERAL NOTES:

- 1. Construct sidewalks in accordance with Specification 522. Use 6" concrete for Sidewalks and Curb Ramps Located within Curb Returns (See Plan View). Install all other concrete with thickness as shown, unless otherwise detailed in the Plans.
- 2. Include detectable warnings on sidewalk curb ramps in accordance with Index 522-002.
- 3. For Driveways see Index 522-003.
- 4. Bond breaker material can be any impermeable coated or sheet membrane or preformed material having a thickness of not less than 6 mils and not more than 1/3".
- 5. Construct sidewalks with Edge Beam through the limits of any surface mounted Pedestrian/Bicycle Railing or Pipe Guiderail shown in the plans. (See RAILING DETAIL)



### OPEN JOINTS



SAWED JOINTS

# LONGITUDINAL SECTION

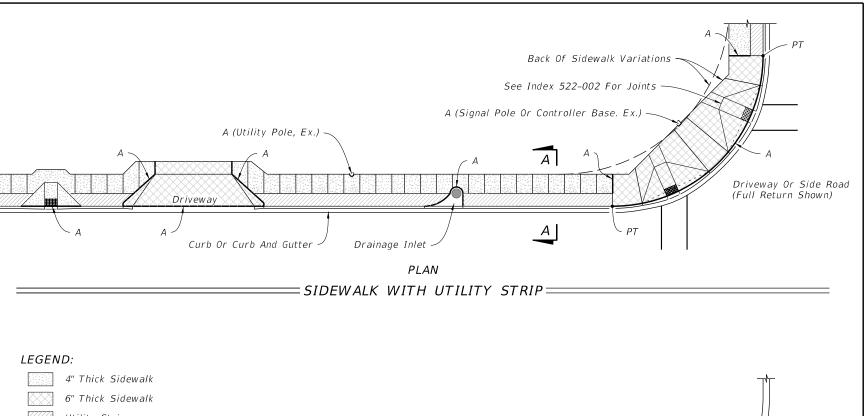
# LEGEND:

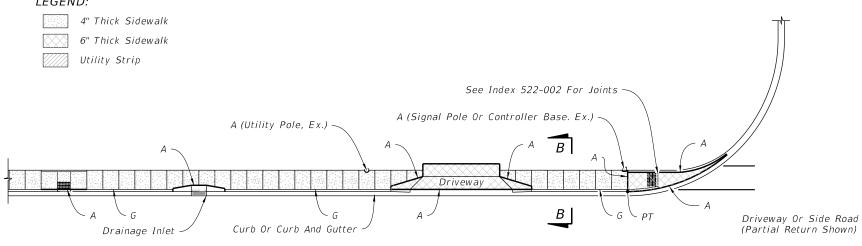
- A- 1/2" Expansion Joints (Preformed Joint Filler) between the sidewalk and; driveways, sidewalk-intersections, and all other fixed objects (e.g. drainage inlets and utility poles).
- B- 1/8" Dummy Joints, Tooled
- C- 1/8" Formed Open Joints

DESCRIPTION:

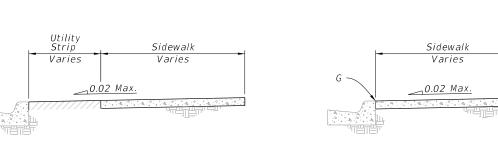
- D- ¾₁₆" Saw Cut Joints, 1½" Deep (within 96 hours) Max. 5' Centers
- $E \frac{3}{16}$ " Saw Cut Joints,  $1\frac{1}{2}$ " Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'
- $F- \frac{1}{2}$ " Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by
- G- Cold Joint With Bond Breaker, Tooled







PLAN SIDEWALK WITHOUT UTILITY STRIP=





Clear Width (5' or 6' Std., 4' Min) <u>__0.02</u> Max. Sidewalk Varies Based on Railing Used

Railing (See Index 515-052, 515-062, 515-070 Or 515-080)

SECTION A-A=

= SECTION B-B=====

=== RAILING DETAIL ====

GENERAL NOTES AND CONCRETE SIDEWALK ON CURBED ROADWAYS

LAST **REVISION** 11/01/18

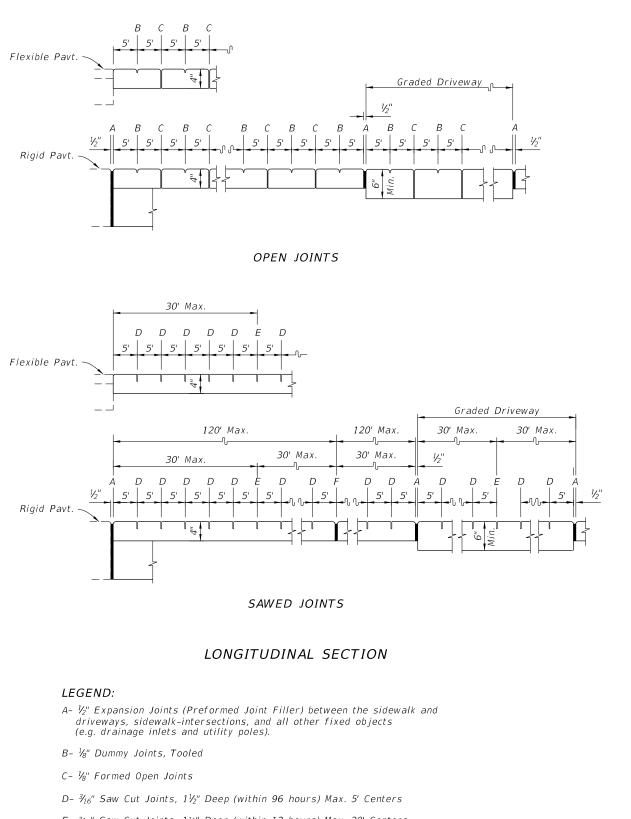
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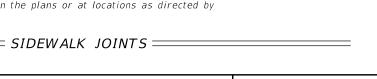
CONCRETE SIDEWALK

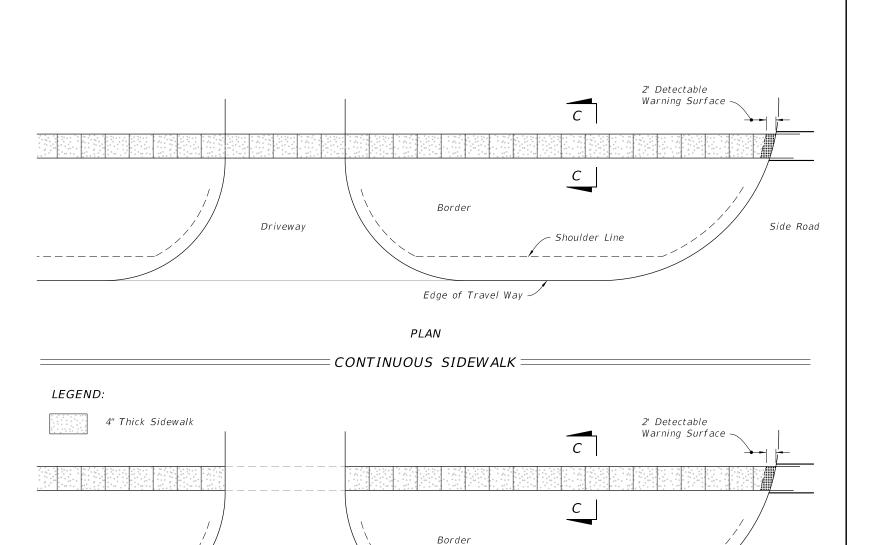
INDEX 522-001

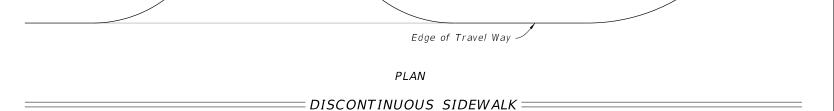
SHEET 1 of 2

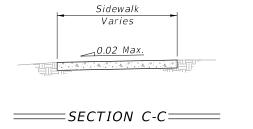


- E- ¾₁₆" Saw Cut Joints, 1½" Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'
- F- 1/2" Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.









CONCRETE SIDEWALK ON FLUSH SHOULDER ROADWAYS

Shoulder Line

LAST **REVISION** 11/01/18

FDOT

FY 2019-20 STANDARD PLANS

CONCRETE SIDEWALK

INDEX *522-001* 

SHEET

Side Road

DESCRIPTION:

Driveway

### GENERAL NOTES:

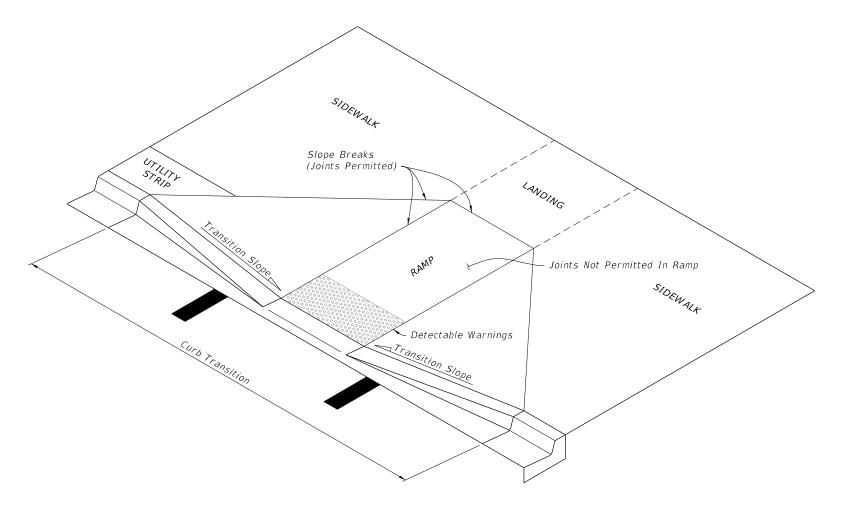
## 1. Cross Slopes and Grades:

- A. Sidewalk, ramp, and landing slopes (i.e. 0.02, 0.05, and 1:12) shown in this Index are maximums. With approval of the Engineer, provide the minimum feasible slope where the requirements cannot be met.
- B. Landings must have cross-slopes less than or equal to 0.02 in any direction.
- C. Maintain a single longitudinal slope along each side of the curb ramp. Ramp slopes are not required to exceed 15 feet in length.
- D. Joints permitted at the location of Slope Breaks. Otherwise locate joints in accordance with Index 522-001. No joints are permitted within the ramp portion of the Curb Ramp.

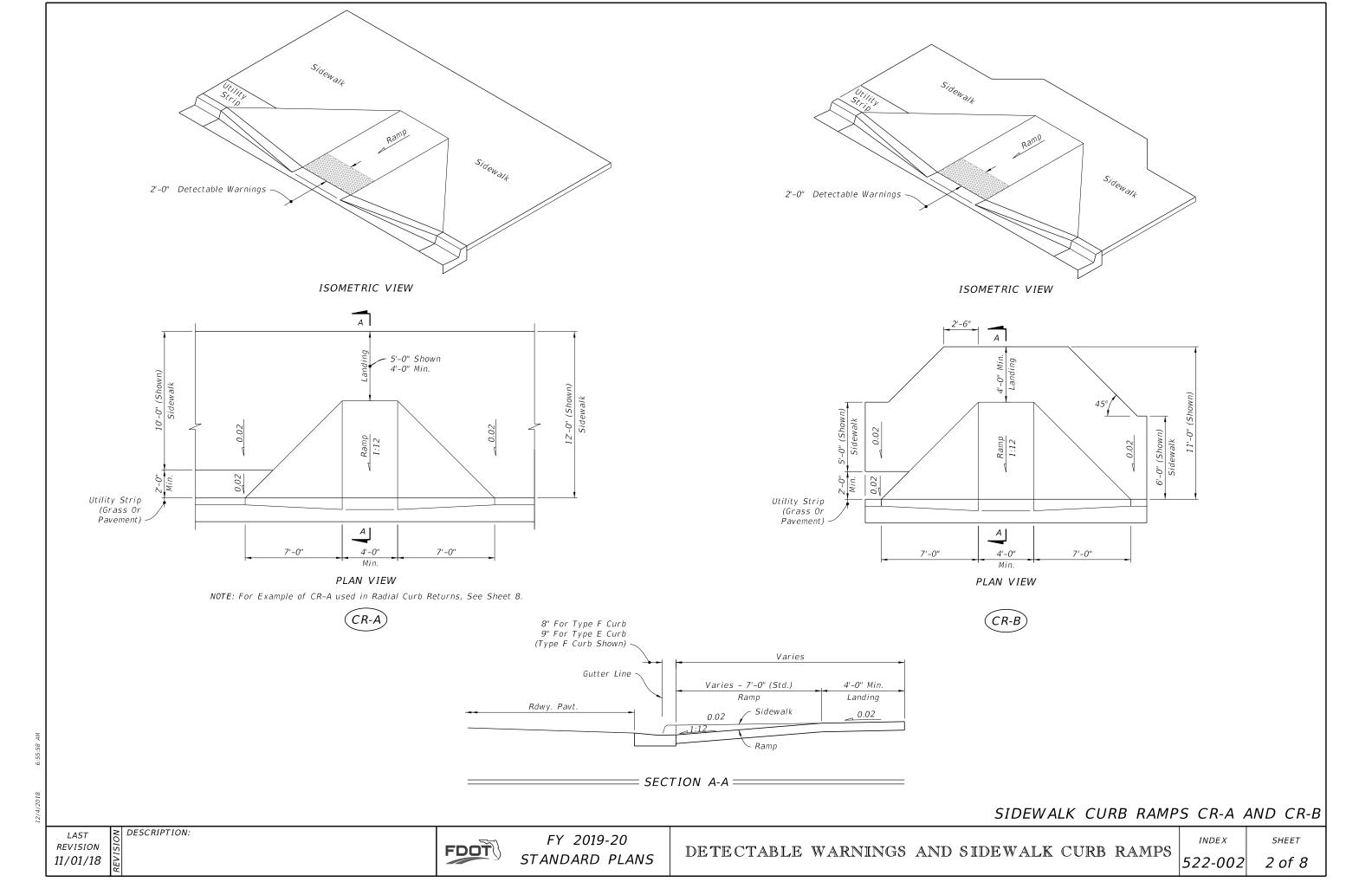
## 2. Grade Breaks:

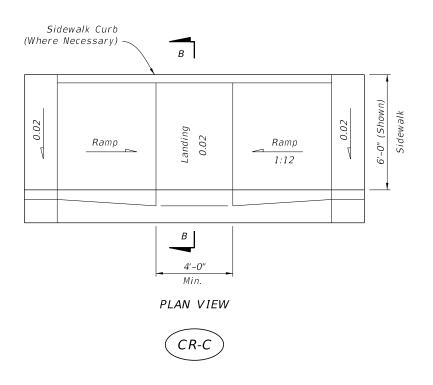
Grade breaks at the top and bottom of ramps must be parallel to each other and perpendicular to the direction of the ramp slope.

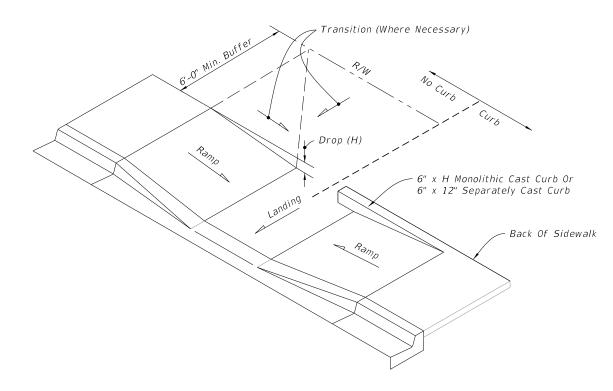
- 3. Curb, Curb and Gutter and/or Sidewalk:
  - A. Refer to Index 522-001 for concrete thickness and sidewalk details.
  - B. Remove any existing curb, curb and gutter, or sidewalk to the nearest joint beyond the curb transition or to the extent that no remaining section is less than 5 feet long.
- 4. Curb Ramp Alpha-Identification:
- A. Sidewalk curb ramp alpha-identifications (e.g. CR-A) are provided for reference purposes in the Plans.
- B. Alpha-identifications CR-I and CR-J are intentionally omitted.
- 5. Detectable Warnings:
  - A. Install detectable warnings in accordance with Specification 527.
  - B. Place detectable warnings across the full width of the ramp or landing, to a minimum depth of 2 feet measured perpendicular to the curb line and no greater than 5 feet from the back of the curb or edge of pavement.
  - C. If detectable warnings are shown in the Plans on slopes greater than 5%, align the truncated domes with the centerline of the ramp; otherwise, the truncated domes are not required to be aligned.
- 6. Detectable Warnings Acceptance Criteria:
  - A. Color and texture shall be complete and uniform.
  - B. 90% of individual truncated domes shall be in accordance with the Americans with Disabilities Act Standards for Transportation Facilities, Section 705.
  - C. There shall be no more than 4 non-compliant domes in any one square foot.
  - D. Non-compliant domes shall not be adjacent to other non-compliant domes.
  - E. Surfaces shall not deviate more than 0.10" from a true plane.



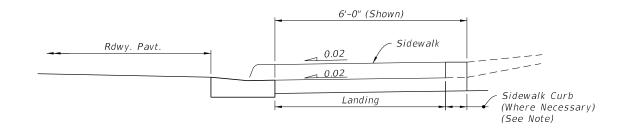
= CURB RAMP NOMENCLATURE =





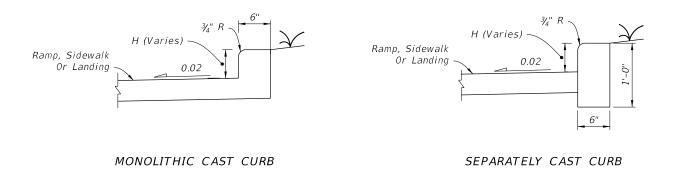


CONSTRUCTION OF SIDEWALK CURB IN CUT SECTIONS



NOTE: For additional information on sidewalk curb construction, see SIDEWALK CURB OPTIONS details.

=SECTION B-B=



= SIDEWALK CURB OPTIONS=

SIDEWALK CURB RAMPS CR-C AND SIDEWALK CURB

LAST REVISION 11/01/18

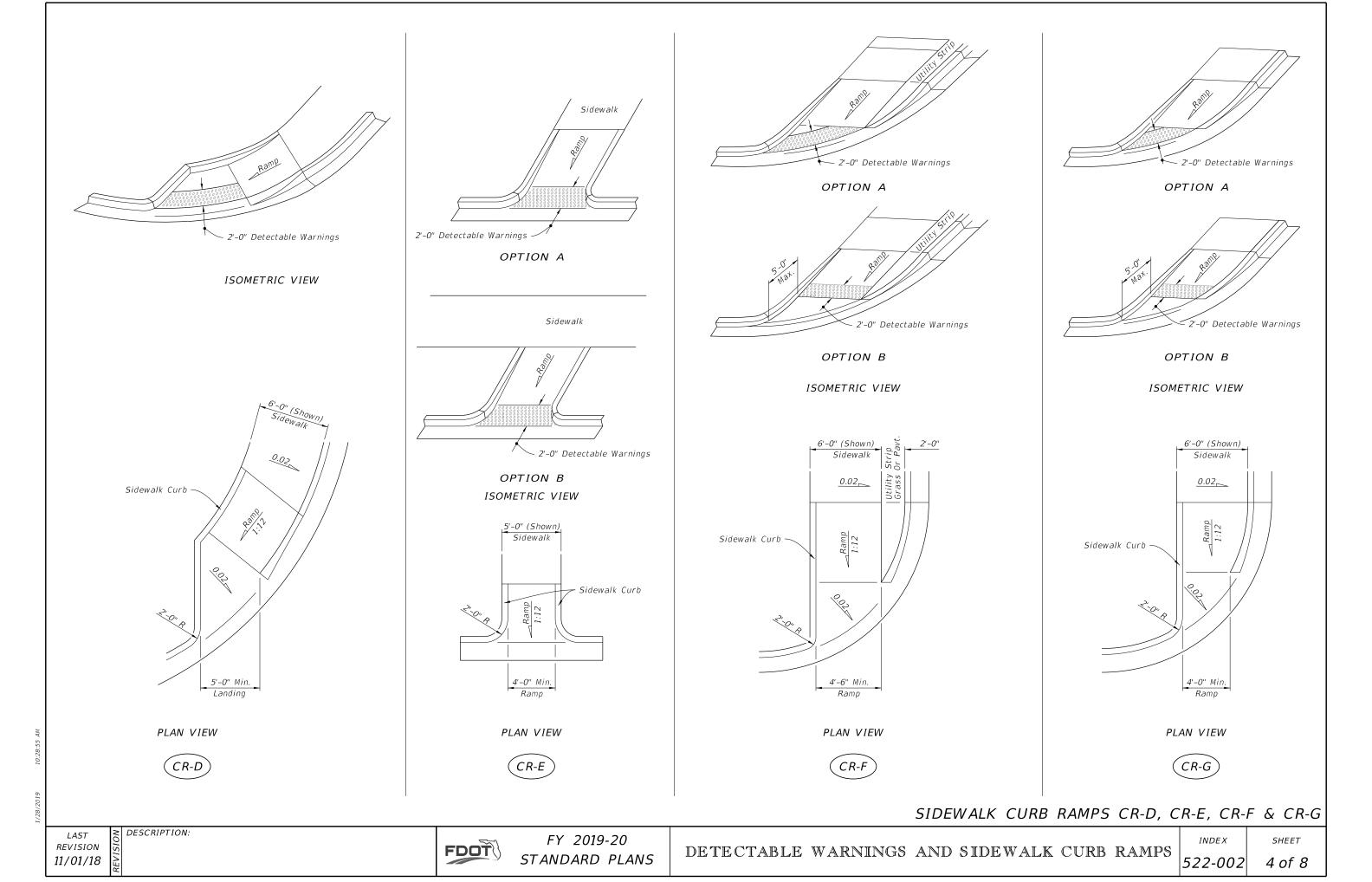
DESCRIPTION:

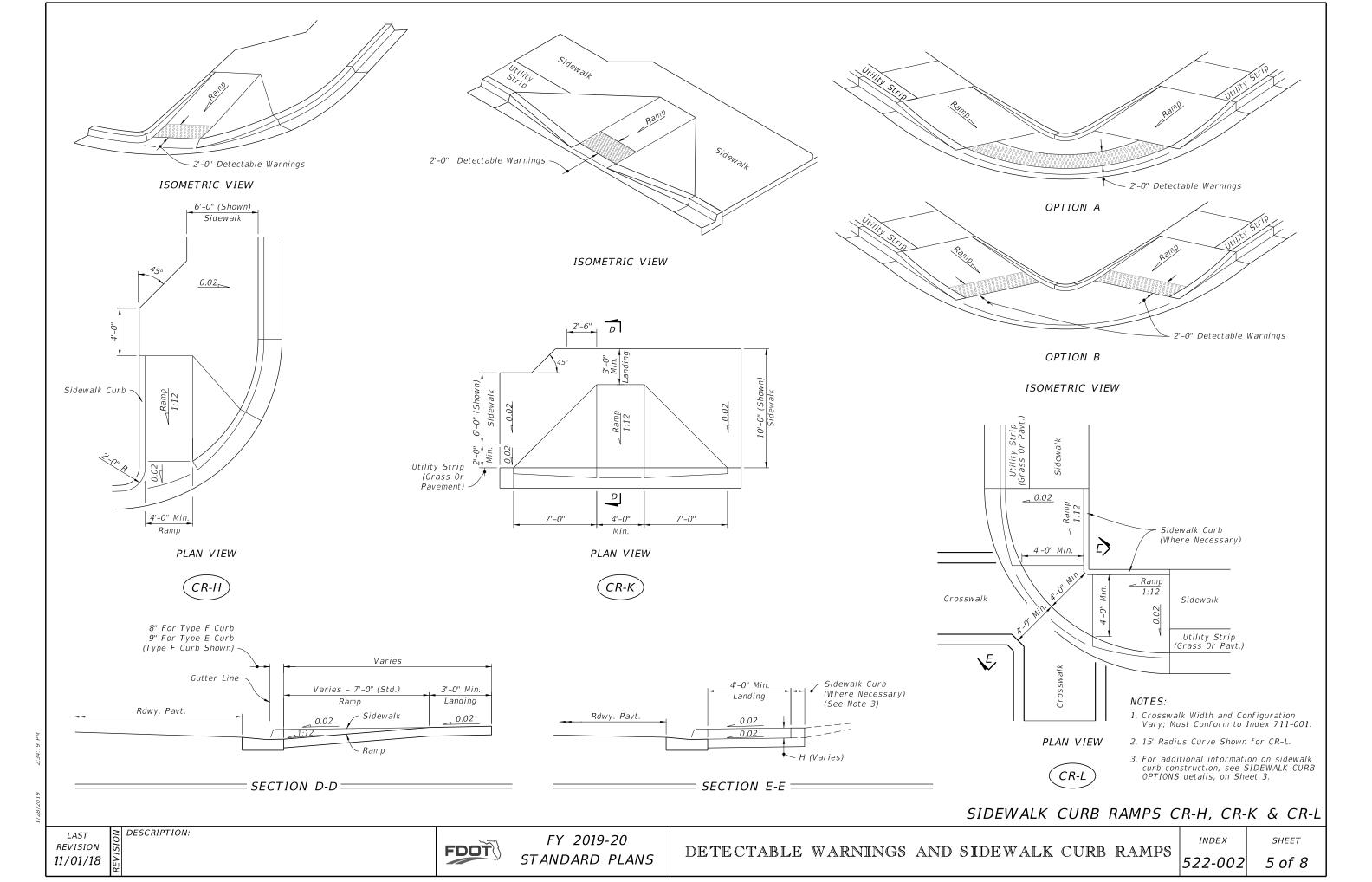
FY 2019-20 STANDARD PLANS

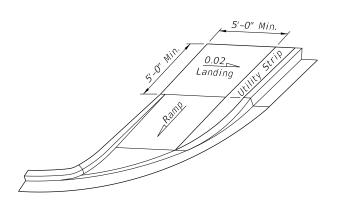
DETECTABLE WARNINGS AND SIDEWALK CURB RAMPS

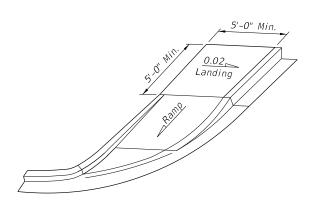
522-002

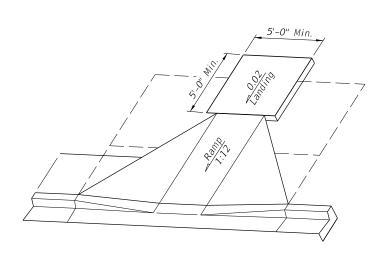
SHEET 3 of 8





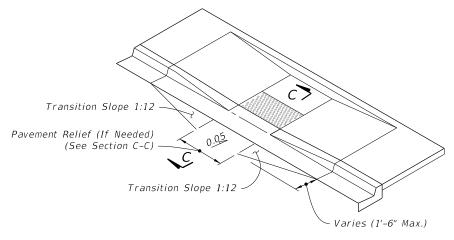






# = LANDINGS FOR CURB RAMPS WITHOUT SIDEWALKS =

(See CR-F, CR-G & CR-K Respectively For Detectable Warning Details/Options)



# ISOMETRIC VIEW (CR-C Shown, Other Similar)

Initial Surface of Pavement Relief

Varies (1'-6" Max.)

Pavement Relief

Ramp Or Landing

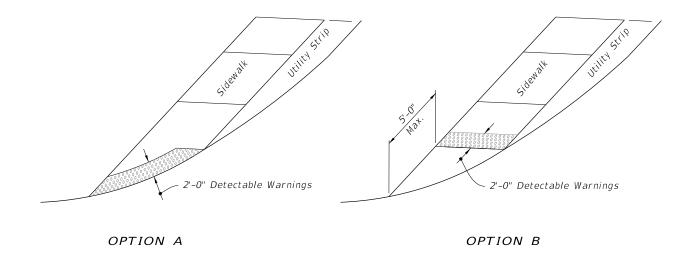
0.05 Max.

Lip Of Curb

NOTE: Remove Elevated Pavement By Spading And Rolling, Smooth Milling, or Grinding.

SECTION C-C

= PAVEMENT RELIEF DETAILS ==



== DETECTABLE WARNING ON FLUSH SHOULDER SIDEWALKS =

CURB RAMPS WITHOUT SIDEWALKS AND FLUSH SHOULDER SIDEWALKS

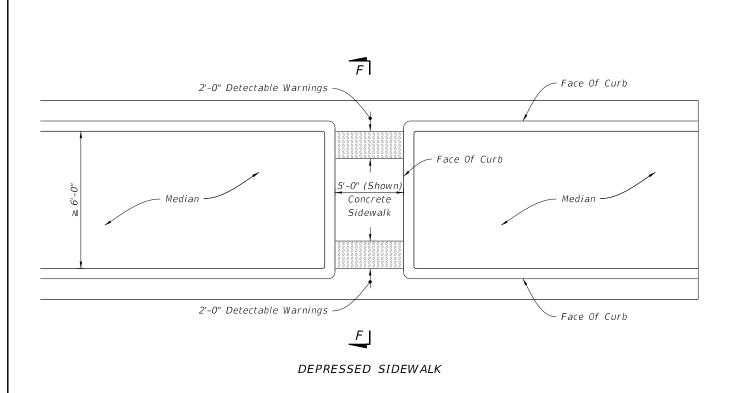
LAST REVISION 11/01/18

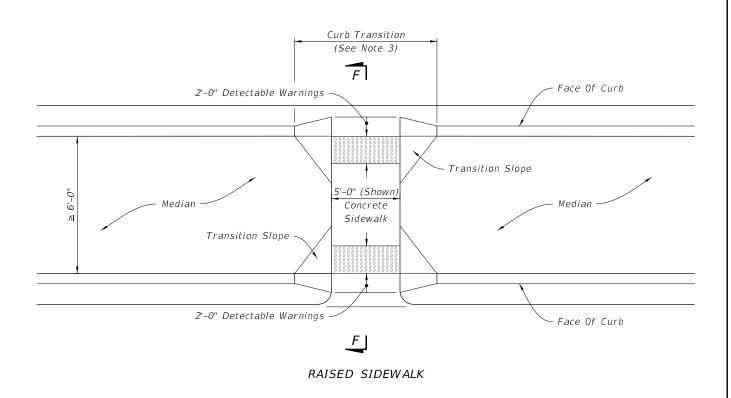
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

SHEET

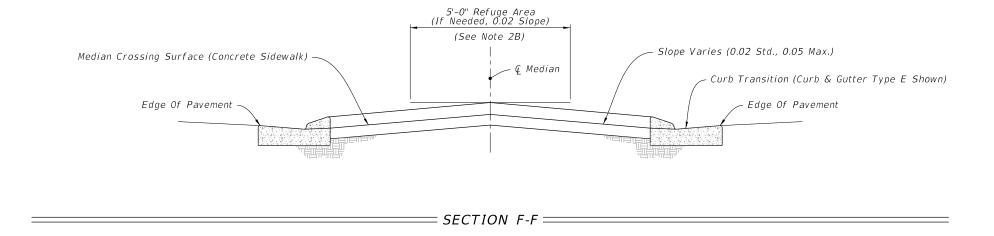




= MEDIAN CROSSINGS =

# NOTES:

- 1. Cross Slope of the median crossing not to exceed 0.02.
- 2. Running Slopes:
  - A. Slopes ≤ 0.05: For roadway cross sections were the Edge of Pavement elevation is the same for both directions of traffic, the median crossing running slopes (0.02 Typ.) should meet at the centerline of the median. For roadway cross sections with variable Edge of Pavement elevations, or to accommodate other construction in the median, the slopes may intersect off the centerline of the median.
  - B. Slopes > 0.05: Provide a median refuge area (landing, 0.02 slope) for crossings with running slopes > 0.05. The refuge area must extend the full width of the crossing and have a minimum length of 5 feet.
- 3. On existing facilities, remove and reconstruct curb transition for raised sidewalk with ramp.



MEDIAN CROSSING

**REVISION** 11/01/17

DESCRIPTION:

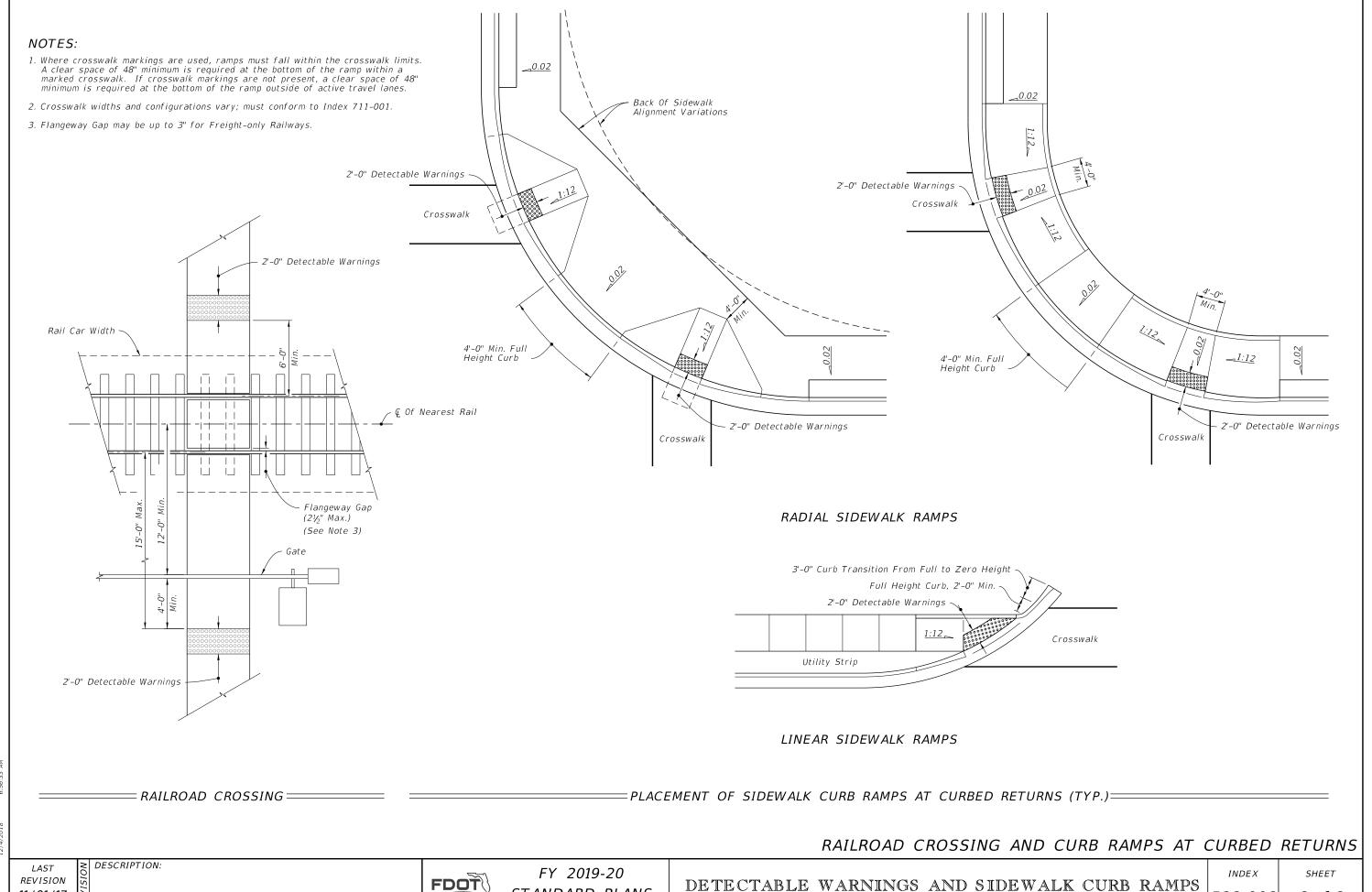
FDOT

FY 2019-20 STANDARD PLANS

DETECTABLE WARNINGS AND SIDEWALK CURB RAMPS

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11/01/17

FDOT

STANDARD PLANS

## **GENERAL NOTES:**

- 1. Work this Index with Specification 522.
- 2. Refer to Index 520-001 for drop curb details and Index 522-001 for joints between driveway, sidewalks, and curb.
- 3. Existing Curb and Gutter:

Remove existing curb and gutter to either the nearest joint beyond the flared point or to where no remaining section is less than 5 feet long.

- 4. Grades and cross slopes shown are maximums.
- 5. <u>Longitudinal Join</u>ts:

Construct  $V_8$  open joints placed at equal (20' max.) intervals for driveways over 20' wide. Match joints in curb and qutter to match joints in driveways.

6. Transverse Joints:

 $\overline{\text{Construct } \mathcal{V}_8'' \text{ open }}$  joints @ 10' Centers and  $\mathcal{V}_2''$  expansion joints with preformed joint filler every 5th joint.

- 7. Construct driveways (6" thick concrete) to a uniform width (W) to the R/W line or the extent shown in the Plans.
- 8. Width of Sidewalk Thru Driveway is 4'-0" minimum. Match sidewalk width when shown in Plans or when utility strip width is equal to or greater than the depth of the Driveway Apron.
- 9. <u>Alpha-Numeric Identification:</u>

Concrete Flared Driveway Alpha-Numeric Identifications (e.g. G4) are provided for reference purposes in the Plans.

# LEGEND:

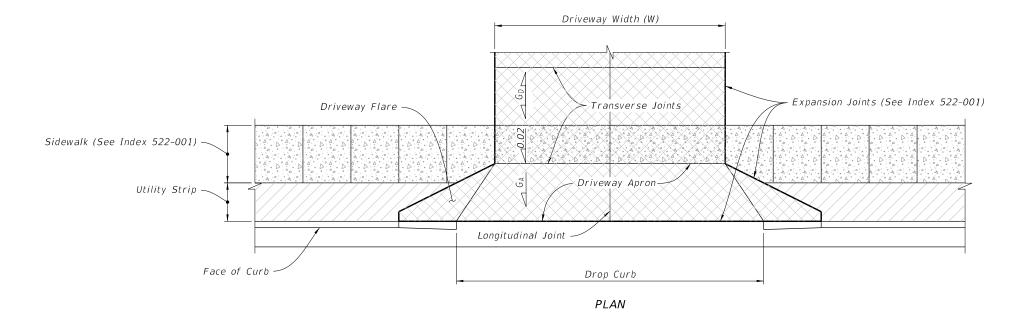
Sidewalk

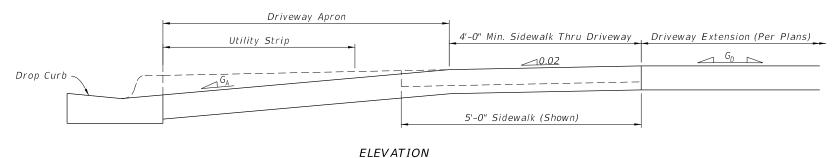
Flared Driveway (6" Thick Concrete)

Sidewalk Thru Driveway (6" Thick Concrete)

Utility Strip

- G_A Grade of Apron
- G_D Grade of Driveway (Per Plans)





_

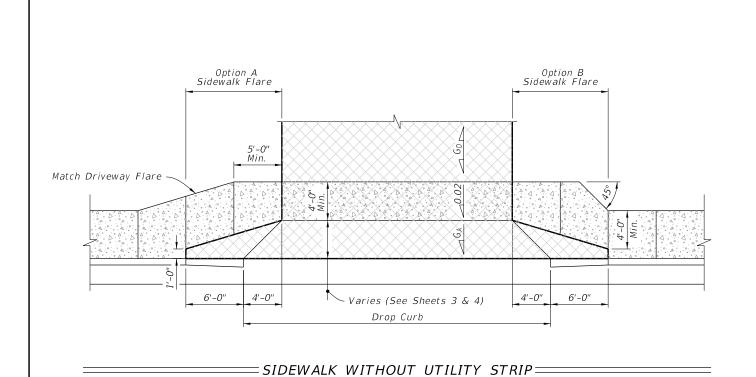
CONCRETE FLARED DRIVEWAY NOMENCLATURE =

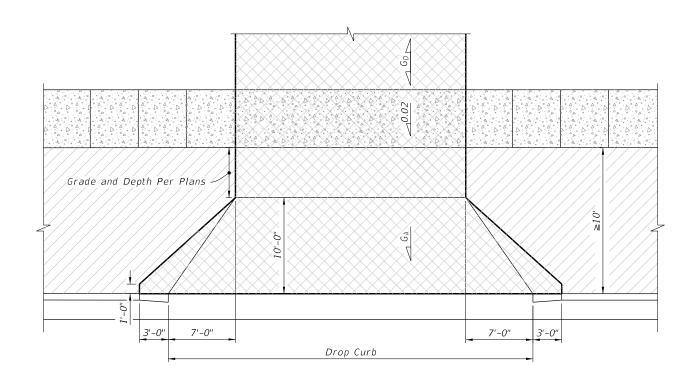
B 1.50.21 B

LAST REVISION 11/01/18

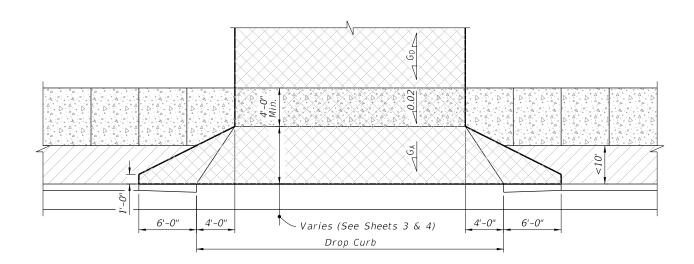
DESCRIPTION:

1 of 4





=WITHOUT SIDEWALK OR UTILITY STRIP ≥ 10' WIDE ===



=UTILITY STRIP < 10' WIDE===

LEGEND:

Sidewalk

Flared Driveway (6" Thick Concrete)

Sidewalk Thru Driveway (6" Thick Concrete)

Utility Strip

REVISION 11/01/18

≥ DESCRIPTION:

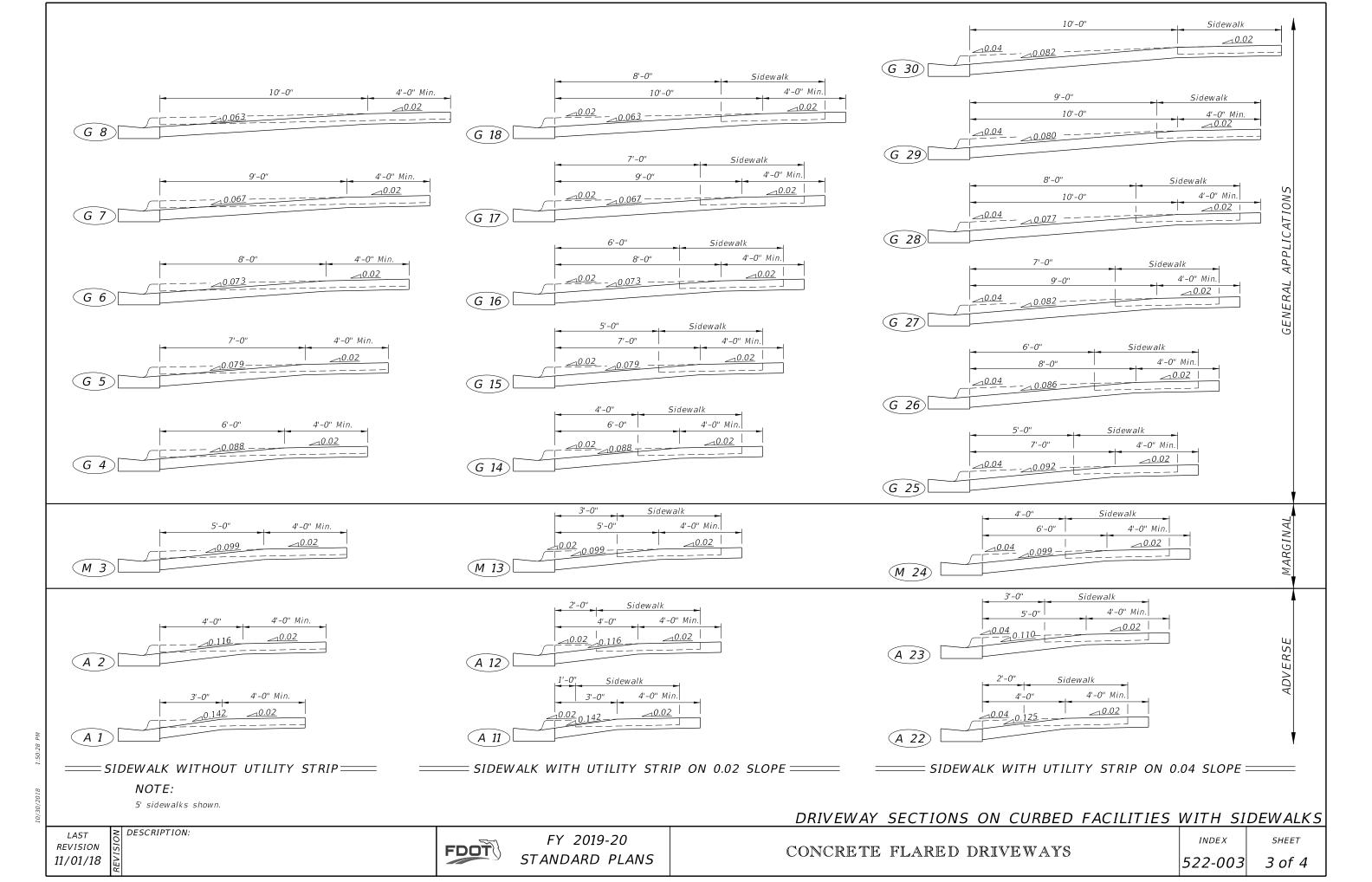
FDOT

FY 2019-20 STANDARD PLANS

CONCRETE FLARED DRIVEWAYS

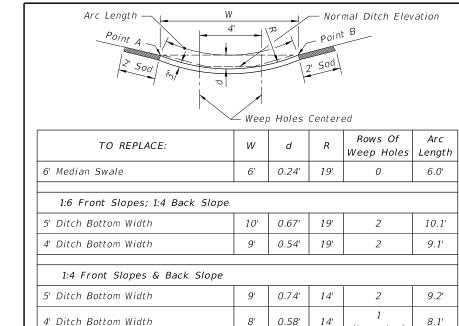
INDEX 522-003

SHEET 2 of 4





0,000



For use only where side slopes are 1:4 or flatter. Point "A" and "B" are to be the same elevation and should be used to locate the paved section.

**≣**5' Min.≣

Do Not Construct Weep Holes In

This Area Or 5' Upstream

- Roadway Ditch

- Back Slope As

Shown On Plans

Ditch Slope

construction.

ALTERNATE DITCH PAVEMENT

Front Slope

JUNCTION OF ROADWAY DITCH*

AND LATERAL DITCH

5' Varies 5'

DESCRIPTION:

Min.

## SECTION MATTING FOR DITCH

**PLAN** 

Matting -

LONGITUDINAL SECTION

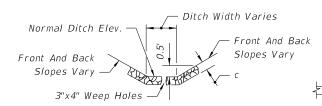
— 6" Overlap

50' Max. Erosion Stops

6" Typical

Front Slope 1:1.5 Slope Shoulder Point © R/W Ditch * Misc. asphalt will not be 5' Min.≡ permitted for this type of

> JUNCTION OF R/W DITCH* AND LATERAL DITCH



TYPICAL SECTION

SECTION AA PROFILE OF DITCH PAVEMENT

Ditch Grade

3"x4" Weep Holes

AT LOCATIONS OTHER THAN JUNCTION WITH LATERAL DITCH

(in center)

					TABLE 1:	DITCH PA	<i>AVEMENT</i>	
Pavement Type	Din	nensi	ons	Payment	Basis Of	Filter Fabric	Velocity	References & Remarks
raveillellt Type	а	b	С	Unit	Estimate	Туре	Range	hererences & hernarks
Concrete	24"	6"	3"	5Y	SY	D-4*	Low-High	Specification 524
Miscellaneous Asphalt	24"	12"	4"	TN	0.2 TN/SY	None	Low-Moderate	Specification 339
Riprap (Sand-Cement)	24"	12"	4"	CY	0.11 CY/SY	D-4*	Low-Moderate	Specification 530, Grouting of joints required
Riprap (Ditch Lining)				TN	TN	D-2*	Moderate-High	Specification 530

Note: All weep holes to be 3"x4" rectangle or 4" or 5" dia. circle hole.  $V_2$  cu. ft. (12" x 12" x 6") of No. 6 aggregate to be placed under each hole. 1 sq. ft. of galv. wire mesh ( $\frac{1}{4}$ " openings) shall be placed between the aggregate and the ditch pavement. Cost of holes, aggregate and wire mesh to be included in the cost of ditch navement.

10' C. to C.

Staples Not More Than 3' Centers

6" Min. Overlap

One Row Of Staples

Each Edge Of Overlaps,

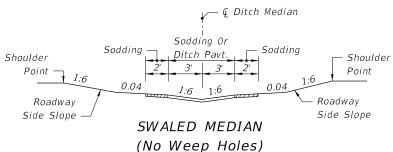
Each Side Of Stops And

On Outer Edges At Not

More Than 18" Centers

(Typical)

#### WEEP HOLE ARRANGEMENT



When Width Is Greater Than 4',

One Row

When "x"= 1' To 4' Const. 1 Row (Centered)

"x"= 5' To 7' Const 2 Rows

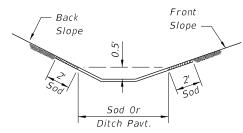
"x"= 8' To 12' Const. 3 Rows

"x"= 13' To 17' Const. 4 Rows

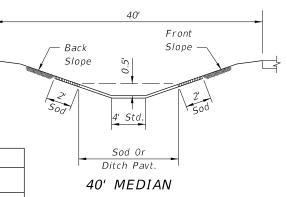
"x"= 18' To 22' Const. 5 Rows

Const. Weep Holes Half-Way Up The

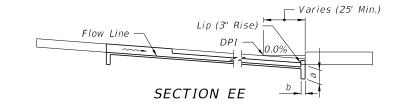
Side In Line With Bottom Weep Holes

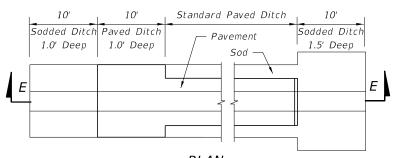


## ROADWAY SIDE DITCH



* Filter Fabric Required.





## PLAN PAVED DITCH END TREATMENT

#### GENERAL NOTES

- 1. Type of ditch pavement shall be as shown on plans.
- 2. In concrete ditch pavement, contraction joints are to be spaced at 25' maximum intervals, or as directed by the Engineer. Contraction joints may be either formed (construction joint) or tooled. No open joints will be permitted in concrete ditch pavement.

Expansion joints with  $\frac{1}{2}$ " preformed joint filler shall be constructed at all inlets, endwalls, and at intervals of not more than 200'.

- 3. Lip at end of ditch pavement shall normally be located downstream of DPI or on flatter grades where there is a decrease in ditch velocity.
- 4. Toewalls are to be used with all ditch paving. A toewall is not required adjacent to drainage structures.
- 5. When directed by the Engineer, weep hole spacing may be reduced to 5' minimum.
- 6. For junction of R/W ditch spillway and lateral ditch, sides of paving to be 1' high minimum.
- 7. For ditch pavements requiring filter fabric (See Table 1) place the filter fabric directly beneath the pavement for the entire length and width of the pavement. See Specification 985 for fabric requirements and application.
- When weep holes with aggregate are used, place filter fabric below the aggregate to form a mat continuous with the pavement filter fabric or underlapping the pavement filter fabric, if present.
- 9. Ditch pavement requiring reinforcement shall be detailed in the plans.
- 10. Cost of plastic filter fabric to be included in the contract unit price for ditch pavement.
- 11. Sodding to be paid for under contract unit price for Performance Turf, SY

**REVISION** 11/01/17

Ditch

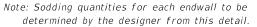
Grade -

**FDOT** 

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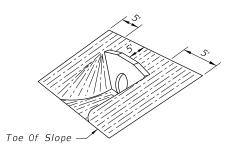
INDEX 524-001

SHEET 1 of 2



(EXCEPT INDEX 430-030)

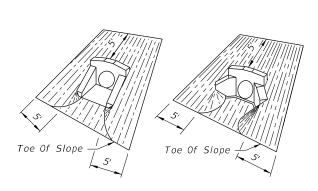
STRAIGHT ENDWALL



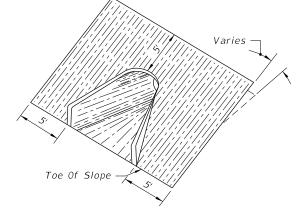
Toe Of Slope

STRAIGHT ENDWALL INDEX 430-030

U-TYPE ENDWALL INDEX 430-011



U-TYPE WINGS 45° WINGS WINGED ENDWALLS INDEX 430-040



FLARED END SECTION INDEX 430-020

	TABLE 2: SOD QUANTITIES (SY)																				
					IN	DEX .	430-0	30					INE	DEX 4	30-011		I	NDEX	430-04	10	INDEX 430-020
PIPE						SLC	PE							SLOF	PE			SLO	OPE		ALL SLOPES
SIZE		1:2			1:3			1:4			1:6		1:2	1:3	1:4	1:6	1:2	1:3	1:4	1:6	ALL SLOTES
						PIP	ES							PIPE	S			PIF	PES		PIPES
	1	2	3	1	2	3	1	2	3	1	2	3	1	1	1	1	1	1	1	1	1
12"																	14	15	18	22	10
15"	19	21	24	22	26	29	26	30	33	34	38	43	13 (15)	16	17	23	15	17	20	25	11
18"	21	24	27	25	29	33	30	34	38	39	44	50	14 (16)	17	19	25	16	18	22	28	11
21"	21"																		12		
24"	24" 26 30 34 32 37 42 38 44 50 50 58										66	15 (17)	19	21	28	19	22	26	34	14	
27"																					15
30"	31	37	42	39	46	53	46	55	63	62	74	85	17 (18)	21	24	32	21	25	30	40	16
36"	37	44	52	46	56	65	56	67	79	76	91	107					24	29	35	47	18
42"	43	53	62	55	67	79	67	82	96	91	111	132					27	32	39	54	19
48"	50	62	73	64	79	93	78	97	115	108	133	158					30	36	44	61	21
54"	57	71	85	74	92	110	91	113	136	126	157	188									21
60"																					22
66"																					25
72"																					26
													() Endwall With Baffles								

## SOD PLACEMENT AT PIPE/CULVERT END TREATMENTS

REVISION 11/01/17

DESCRIPTION:

FY 2019-20 STANDARD PLANS

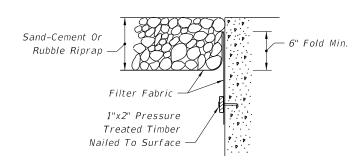


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SHEET 2 of 2

Sand-Cement Or 6" Fold Min. Rubble Riprap No Adhesive Above Here 12" Bituminous Coating On Face Of Concrete

## BONDED OPTION



### NAILED OPTION

Note: Either option may be used unless otherwise called for in the plans.

## FILTER FABRIC PLACEMENT AT CONCRETE STRUCTURE

#### *NOTES*

- 1. Work this Index with the Noise Wall Data Tables, and Wall Control Drawings in the Plans.
  - A. Prestressed concrete posts with equivalent strength resistance may be substituted for conventionally reinforced precast posts shown in this index when approved as part of a Producer's Quality Control Plan.
  - B. Producer shop drawings for prestressed concrete post designs must be approved by the State Structures Design Office prior to inclusion in the Quality Control Plan.
- 2. Construct Noise Walls in accordance with the requirements of Specification Section 534, and Augers Cast Piles in accordance with Specification Section 455.
- 3. Field verify the location of all overhead and underground services shown in the Wall Control Drawings.
- 4. Wall Height is the nominal height of the walls above finished grade. The Wall Embedment Depth for design is 1'-0". The actual embedment depth may vary plus or minus 6" along the length of the wall.
- 5. Post Spacing in this Index are nominal, and are measured from centerline to centerline of the auger cast piles. Actual post spacing may vary as shown in the Wall Control Drawings.
- 6. Panels:
  - A. The sum of the individual stacked panel heights is the Wall Height plus 1'-0" (embedment depth).
  - B. Where special graphics are required, locate the horizontal panel joints outside of the graphics. Where possible, hold horizontal panel joints at a constant elevation.
  - C. Side Installed Panels are only permitted when reduced overhead clearance between posts prohibits installing panels from the top.
    - 1. For Flush Face panels, install panel into posts from the roadway (front face) of the wall. Recessed panels may be installed from the back face of the wall.
    - 2. After panels are installed and centered between posts, grout between both panel ends and the adjoining posts (see Sheets 4 and 5 for details).
  - D. Individual panel heights should be between 6'-0" and 12'-0" tall. The minimum panel height is 4'-0" and may be used where overhead clearance is limited, or where graphic panels are required on shorter walls.
- 7. Concrete And Grout:
  - A. Concrete Class and Compressive Strength for:
    - 1. Precast Panels, Posts, and Post Caps: Class IV
    - 2. Cast-In-Place Collars: Class IV
  - B. Minimum Compressive Strength for form removal and handling of posts and panels:
    - 1. 2,500 psi for horizontally cast post and panels
    - 2,000 psi for vertically cast panels or when tilt-up tables are used for horizontally cast panels.
  - C. Grout for Auger Cast Piles:
    - 1. Maximum Working Compressive Strength = 2,000 psi
    - 2. Minimum 28 day strength = 5,000 psi
- 8. Reinforcing Steel:
  - A. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
    - 1. Post Stirrups Tie at all four corner bars and at every third interior bar intersection.
    - 2. Pile Stirrups Tie to the main vertical reinforcing at alternate intersections for circular configurations and at the four corners and at every third interior bar intersection for rectangular configurations.
  - B. Provide 2" concrete cover unless noted otherwise.
- 9. Casting Tolerances for precast panels and posts:
  - A. Overall Height and Width: +/- 1/4"
  - B. Thickness:  $\pm 1/-\frac{1}{4}$ "
  - C. Plane of side mold: +/- 1/16"
  - D. Openings: +/- 1/2"
  - E. Out of Square: 1/8" per 6 ft., but not more than 3/8"total along any side
  - F. Warping: 1/16" per foot distance to nearest corner
  - G. Bowing: 1/240 panel dimension
  - H. Surface Smoothness for Type "A" Smooth Surface Texture Option: +/- 1/16"

10. Provide Plain or Fiber Reinforced Bearing Pads meeting the requirements of Specification Section 932 for Ancillary Structures.

2. Plain Pads may be substituted for Fiber Reinforced Pads when

sufficient bearing area is available on the concrete collar for the

b. 20' Post Spacing and Wall Height < 17 feet: 4"x 4"x \frac{1}{2}"

c. 20' Post Spacing and Wall Height ≥ 17 feet: 4"x 5"x ½"

B. At panel bearing points between stacked panels, use Plain or Fiber Reinforced

1.  $4"x \ 4"x \ \frac{1}{2}"$  Fiber Reinforced Pads;

a. 10' Post Spacing:  $4''x \ 4''x \ \frac{1}{2}''$ 

A. For Collar Bearing Points provide:

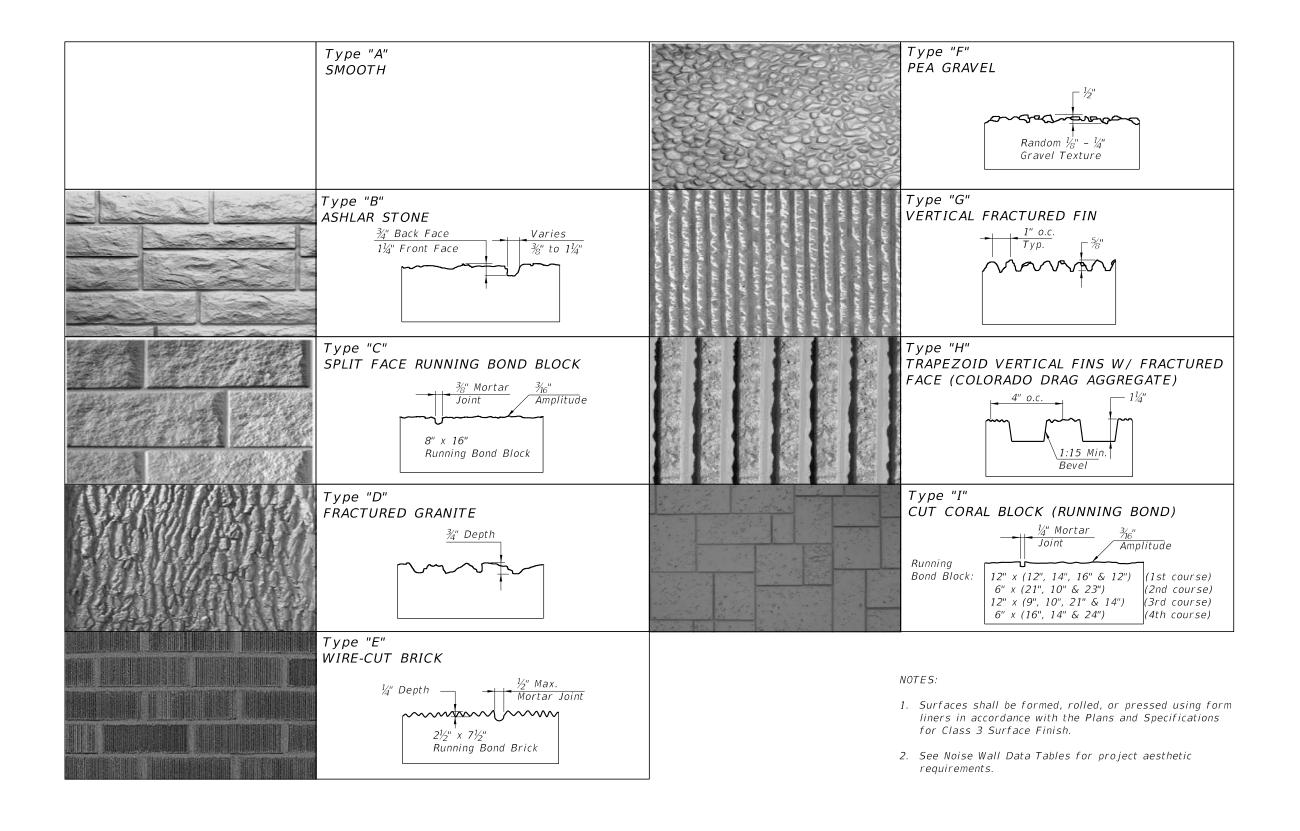
Bearing Pads.

INDEX SHEET

GENERAL NOTES

LAST REVISION 11/01/16

DESCRIPTION:



TEXTURE OPTIONS

REVISION 07/01/13

FDOT

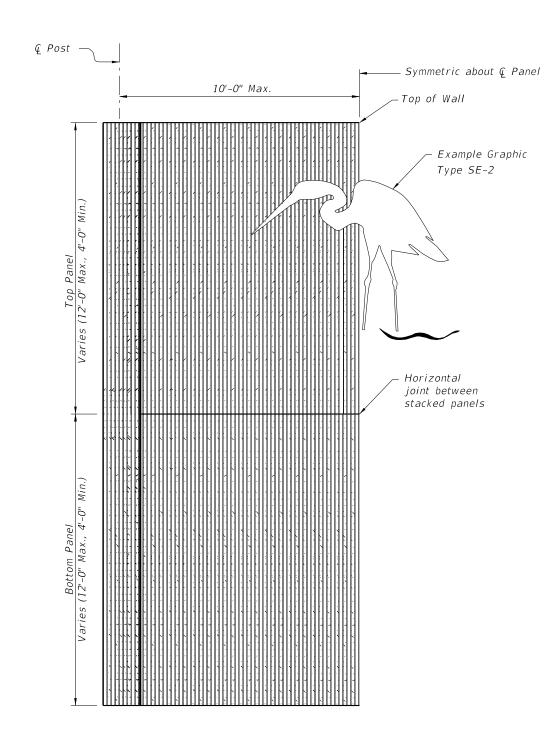
FY 2019-20 STANDARD PLANS

NOISE WALLS - (PRECAST)

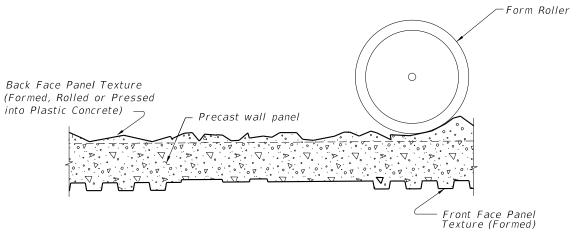
INDEX 534-200

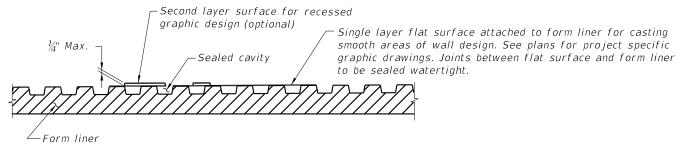
SHEET 2 of 16

DESCRIPTION:



HALF ELEVATION (Front Face Post and Panel Texture Type "H" shown) (Graphic Type SE-2 shown) (Two stacked panels shown, three stacked panels similar)





TYPICAL FORMING DETAIL (Front Face Panel Texture Type "H" shown) (Back Face Panel Texture Type "D" shown) (Post Forming Details Similar)

#### NOTES:

- 1. Submit specific form liner samples for approval by the Engineer.
- 2. Textures and graphics shown are for demonstration purposes only. See Noise Wall Data Tables in the plans for project specific texture and graphic requirements.

GRAPHICS & TEXTURE DETAILS

REVISION 07/01/14

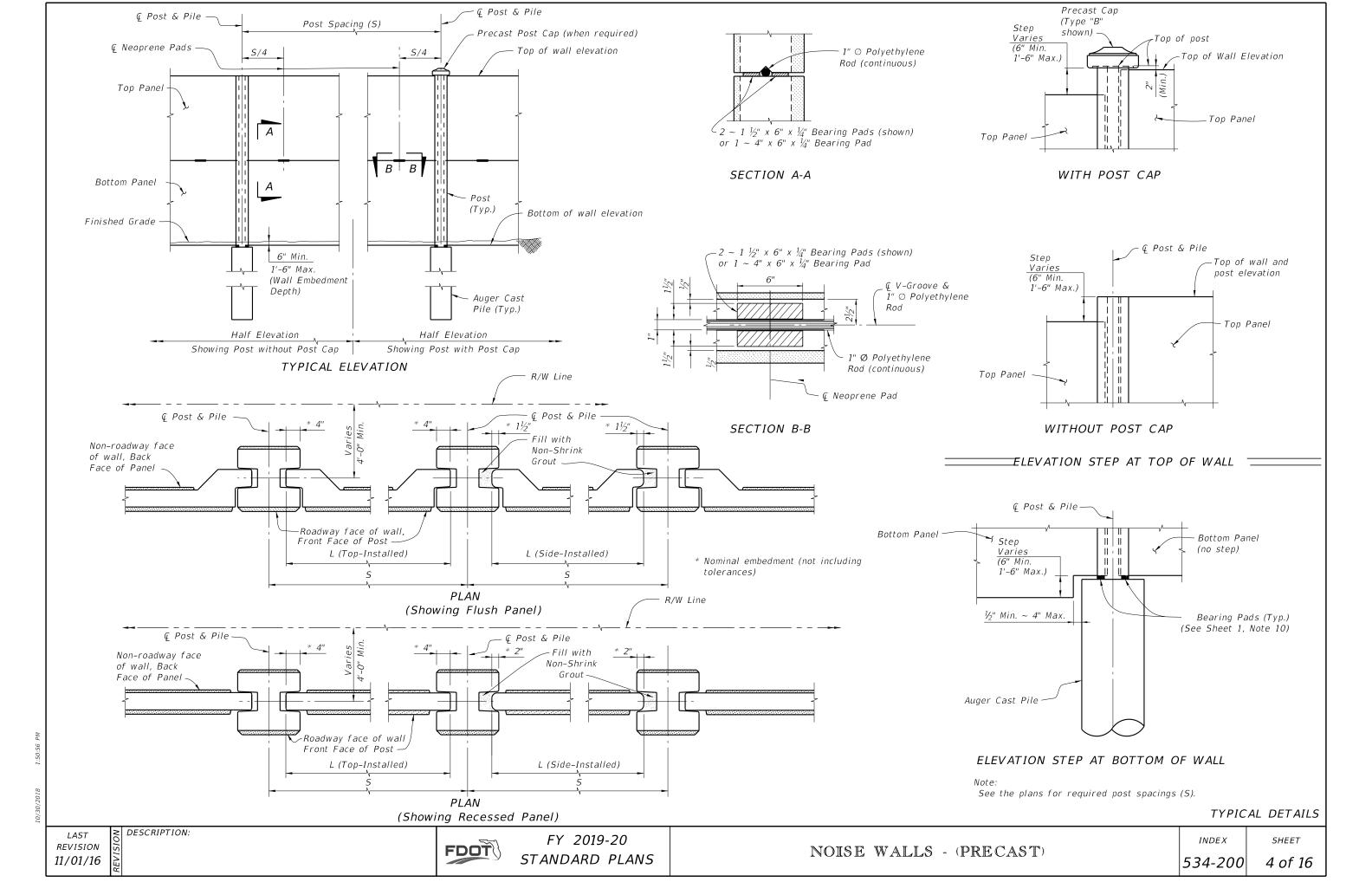
FY 2019-20 STANDARD PLANS

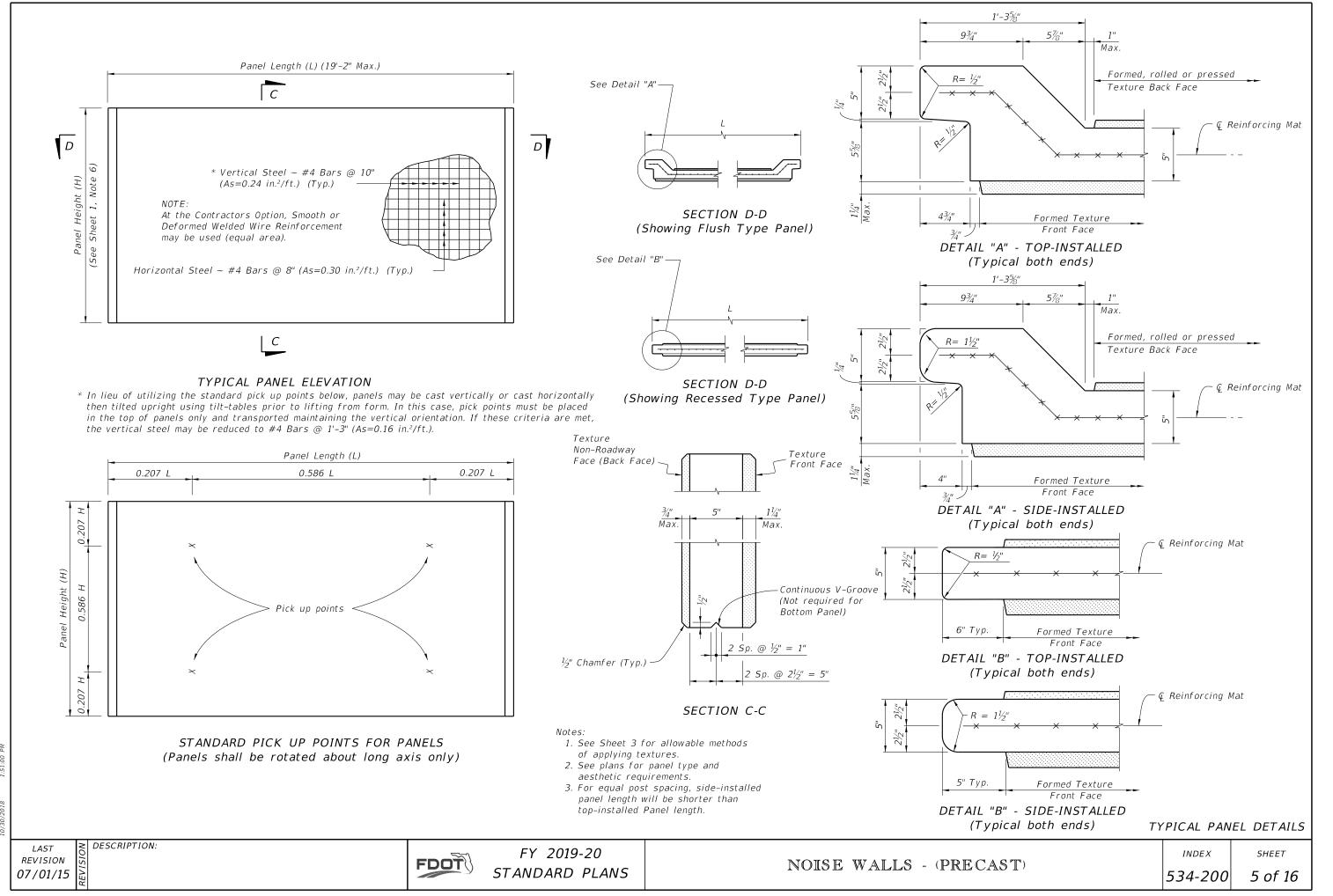
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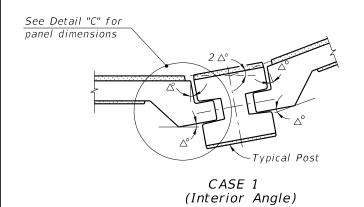
SHEET 3 of 16

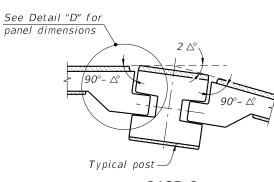
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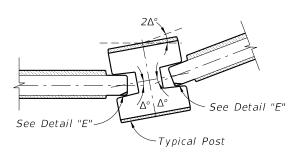
NOISE WALLS - (PRECAST)

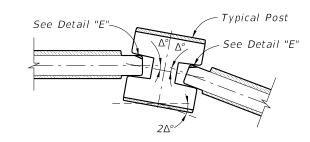








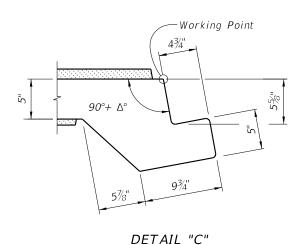


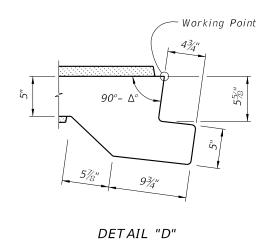


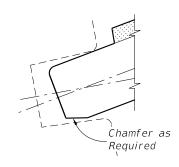
CASE 2 (Exterior Angle)

CASE 1 (Interior Angle)

CASE 2 (Exterior Angle)







DETAIL "E" (Back Face Chamfer Shown Front Face Chamfer Similar)

NOTE.

The shop drawings shall include specific pivoting details of panel ends at locations where the deflection angle ( $2\Delta^{\circ}$ ) between panels exceeds  $7^{\circ}$ .

PIVOTING DETAILS -

(Flush Type Panel)

NOTE:

The shop drawings shall include specific pivoting details of panel ends at locations where the deflection angle  $(2\Delta^\circ)$  between panels exceeds  $20^\circ$ .

PIVOTING DETAILS _______(Recessed Type Panel)

TYPICAL PANEL DETAILS

LAST REVISION

DESCRIPTION:



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NOISE WALLS - (PRECAST)

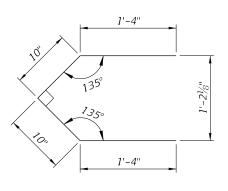
INDEX 534-200

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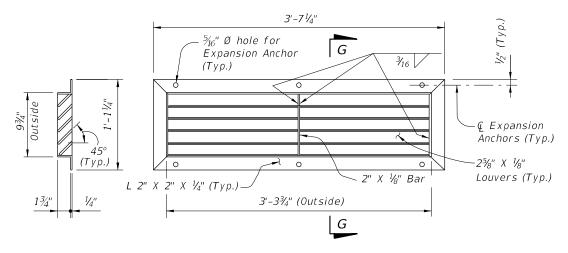
DRAINAGE HOLES TYPES A, B, C & D (Front Face of Wall Shown) (Two Holes Shown, One Hole Similar)

to center of opening. See Wall Control Drawings in the plans.



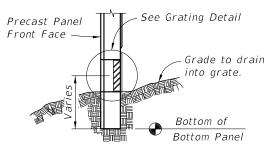
BAR A2 (Pair) Bar Length = 4'-4''

🗆 BAR BENDING DETAILS (#3 Bars) 💳



SECTION G-G

GRATING DETAIL



SECTION F-F

#### GRATING NOTES:

- 1. Grating shall be ASTM A36 steel welded in accordance with the current edition of ANSI/AWS D1.1 Steel Welding Code. Hot-dip galvanize grate after fabrication in accordance with Specification
- 2. Expansion Anchors: Use  $\frac{1}{4}$ " Ø x 2" min. corrosion resistant (zinc/aluminum alloy or stainless steel) expansion anchors to connect grates to panels.
- 3. Blockout textured concrete surface for a strip 2" wide around drainage hole to enable secure attachment of the drainage grate.

DRAINAGE HOLE DETAILS

**REVISION** 11/01/17

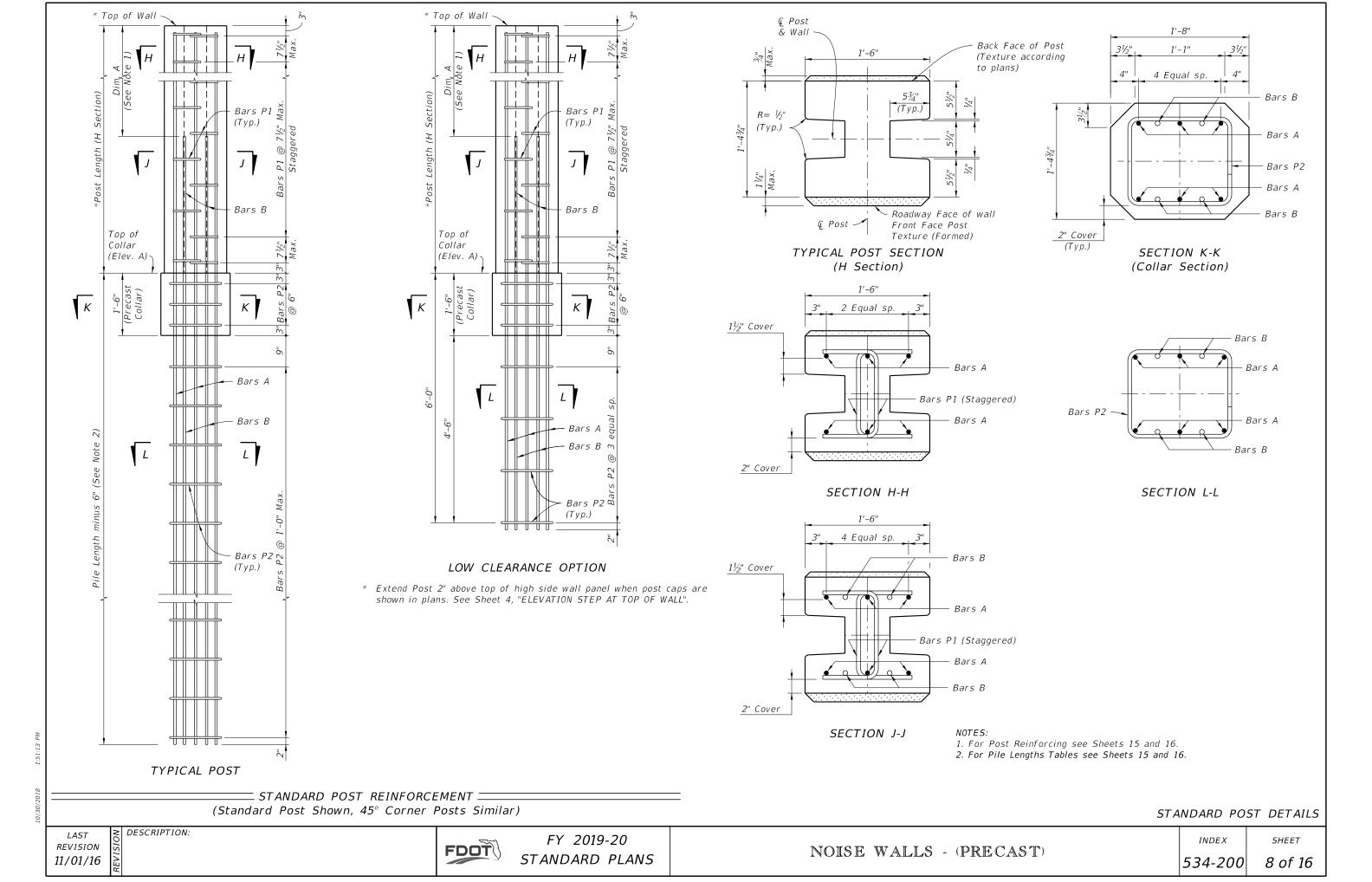
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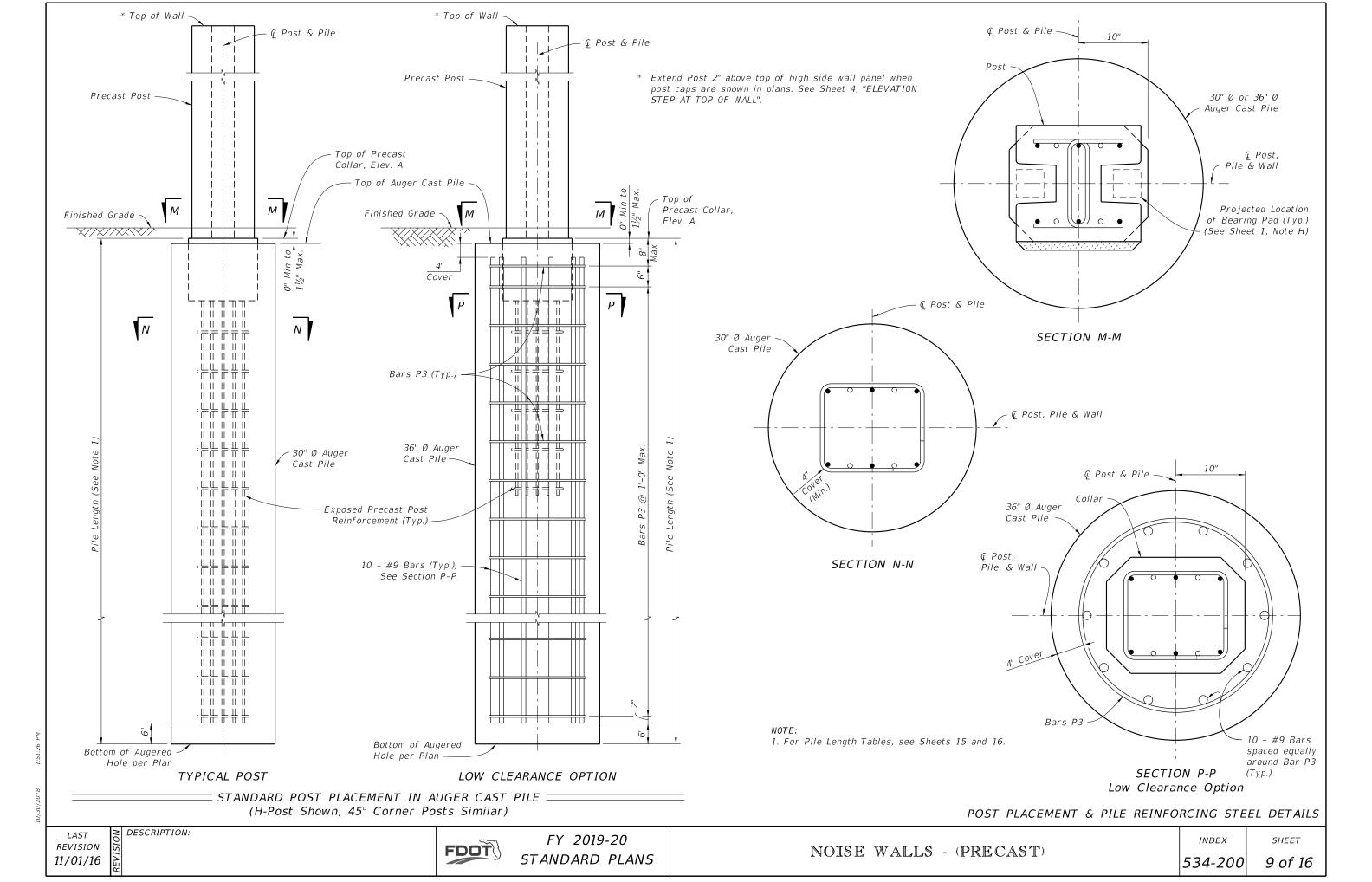
FDOT

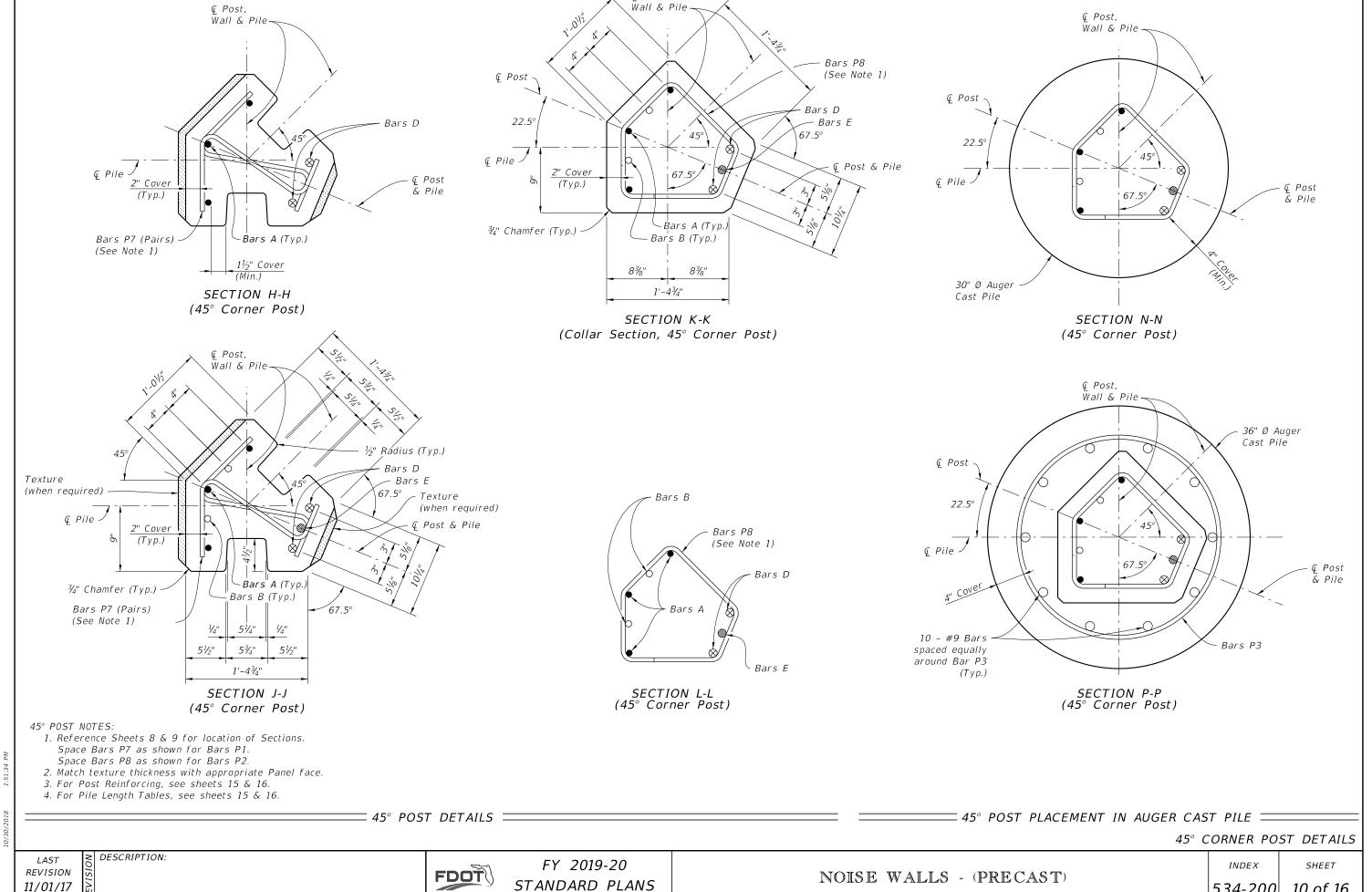
FY 2019-20 STANDARD PLANS

NOISE WALLS - (PRECAST)

INDEX SHEET 534-200



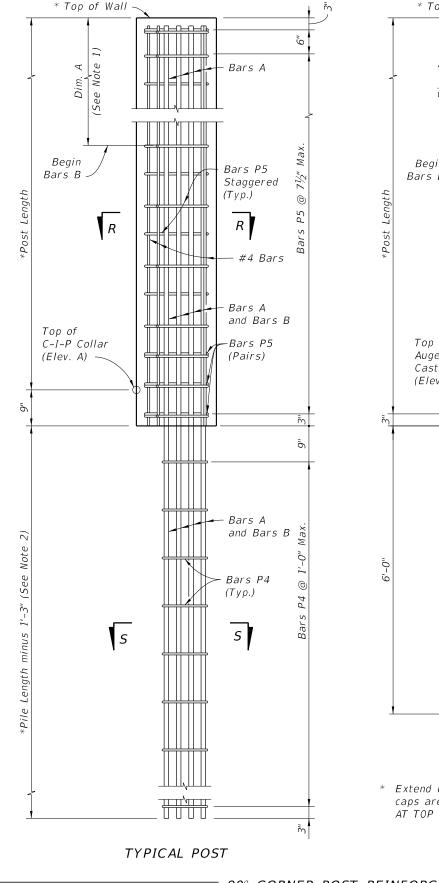


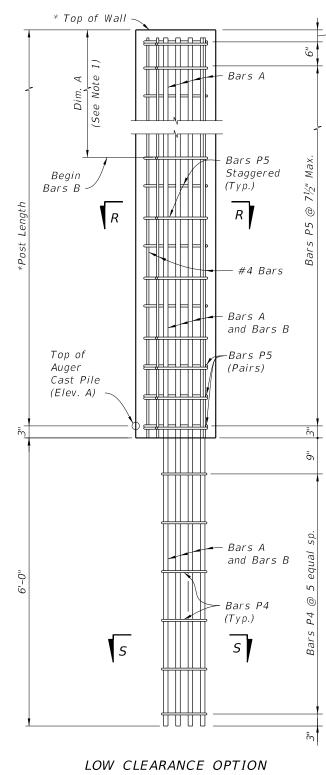


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€ Post,

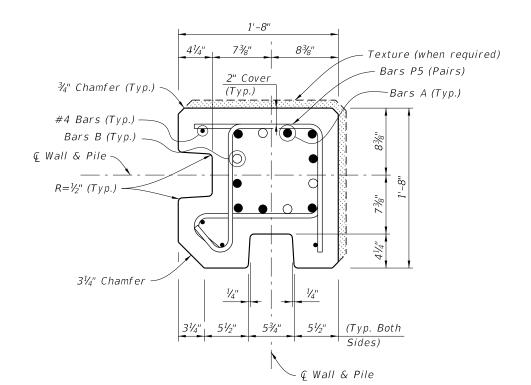
11/01/17



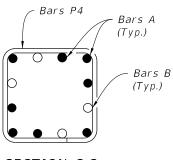


* Extend Post 2" above top of high side wall panel when post caps are shown in plans. See Sheet 4, "ELEVATION STEP AT TOP OF WALL".

______ 90° CORNER POST REINFORCMENT _____ (Post Surface Features Not Shown For Clarity)



#### SECTION R-R



SECTION S-S

#### 90° CORNER POST NOTES:

- 1. For Post Reinforcing, see Sheets 15 and 16.
- 2. For Pile Length Tables, see Sheets 15 and 16.
- 3. Reduce typical panel length or adjust pile spacing at each 90° Corner Post.
- 4. Match texture thickness with appropriate Panel face.

90° CORNER POST DETAILS

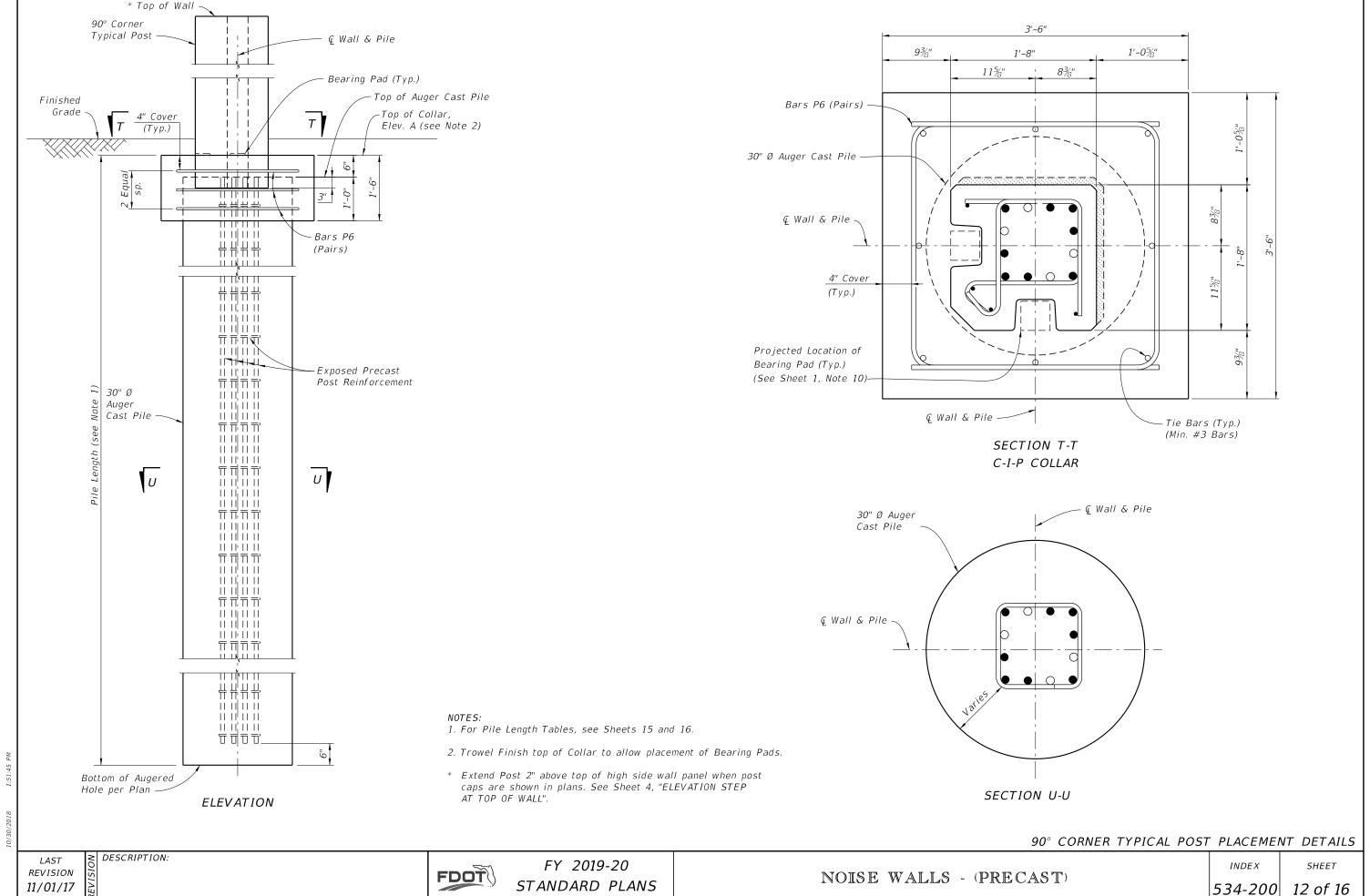
LAST REVISION 11/01/16

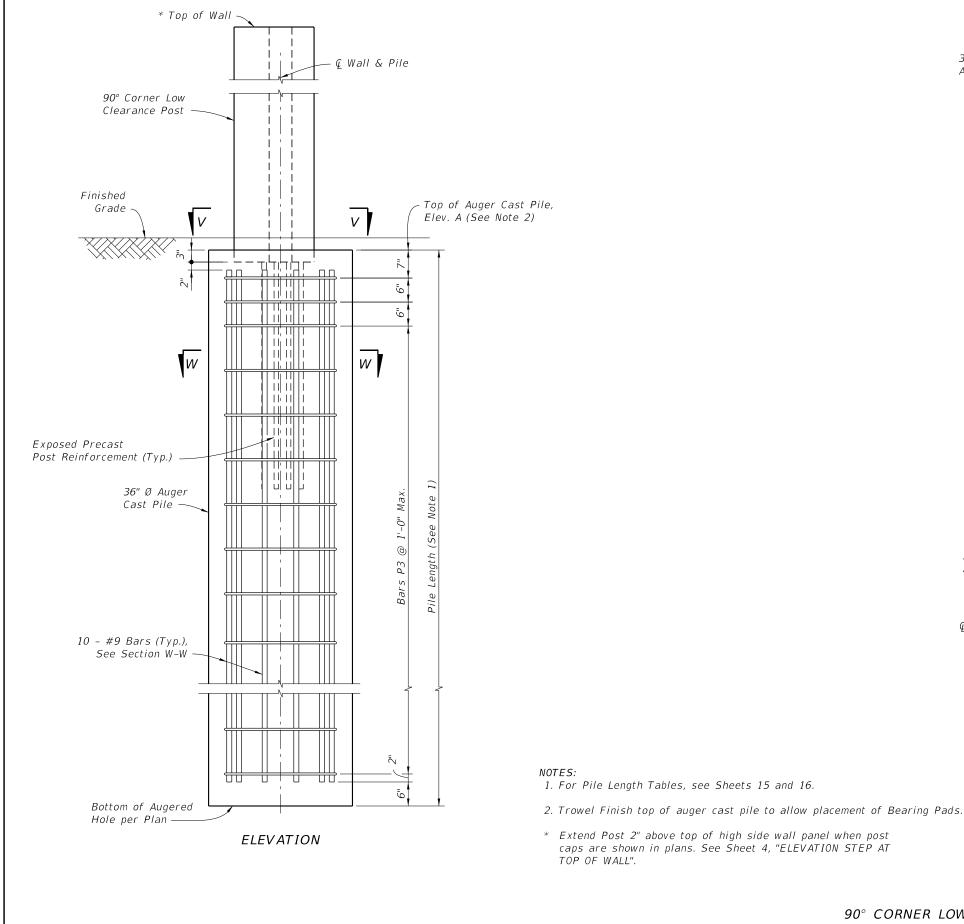
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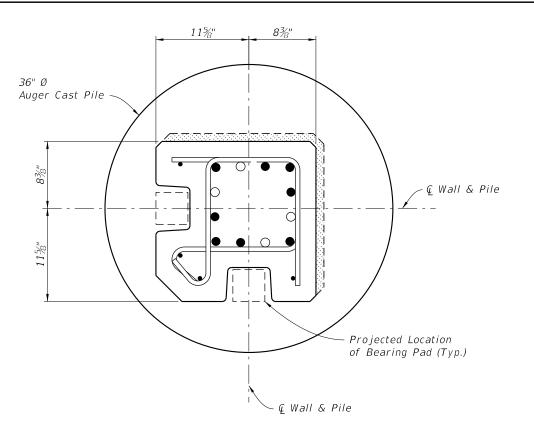
FDOT

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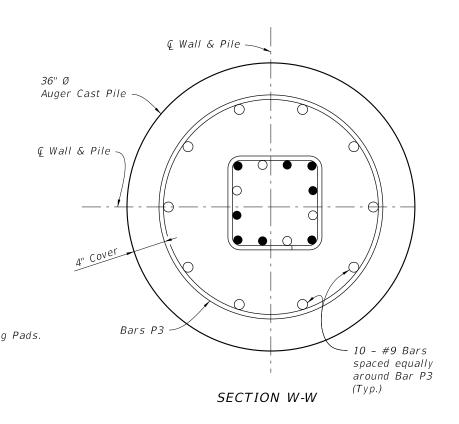
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SECTION V-V



90° CORNER LOW CLEARANCE POST PLACEMENT & PILE REINFORCING STEEL DETAILS

DESCRIPTION: REVISION 07/01/12

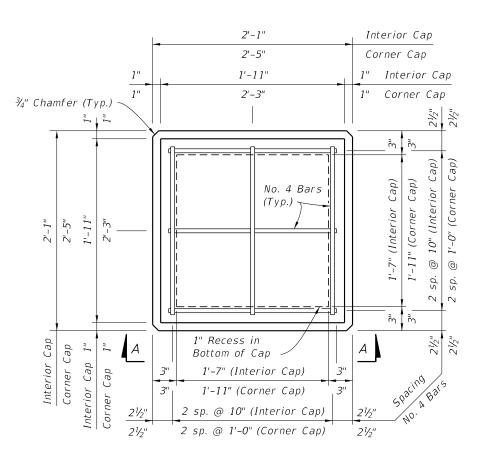
FDOT

FY 2019-20 STANDARD PLANS

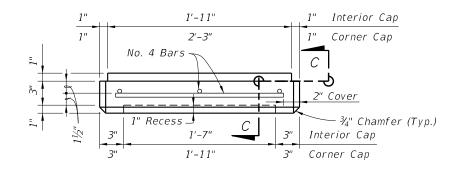
NOISE WALLS - (PRECAST)

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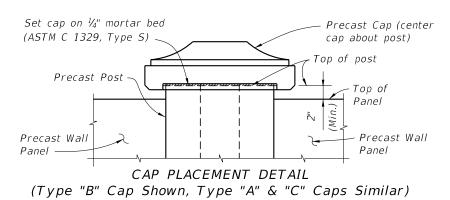
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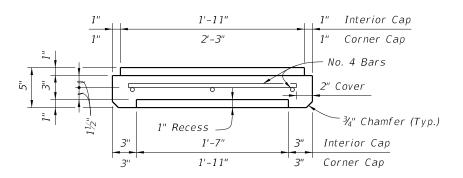


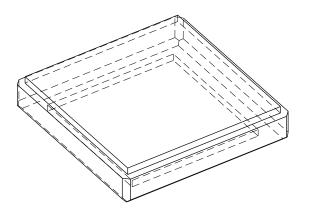
PLAN VIEW (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



VIEW A-A SHOWN, VIEW B-B SIMILAR (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



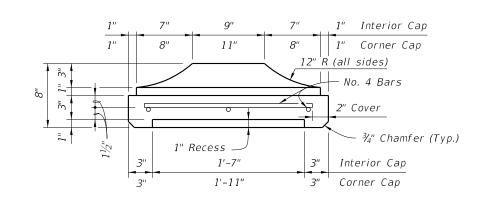


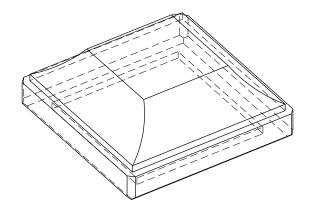


SECTION C-C

PICTORIAL VIEW

= TYPE "A" CAP DETAILS =

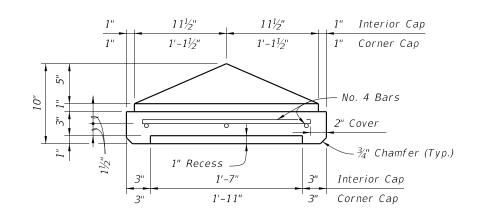


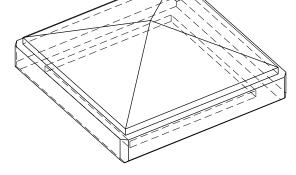


SECTION C-C

PICTORIAL VIEW

TYPE "B" CAP DETAILS =





SECTION C-C

PICTORIAL VIEW

= TYPE "C" CAP DETAILS ==

PRECAST POST CAPITAL

REVISION 07/01/14

DESCRIPTION:

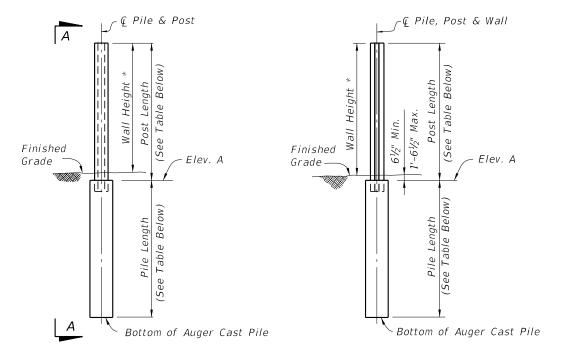
FDOT

FY 2019-20 STANDARD PLANS

NOISE WALLS - (PRECAST)

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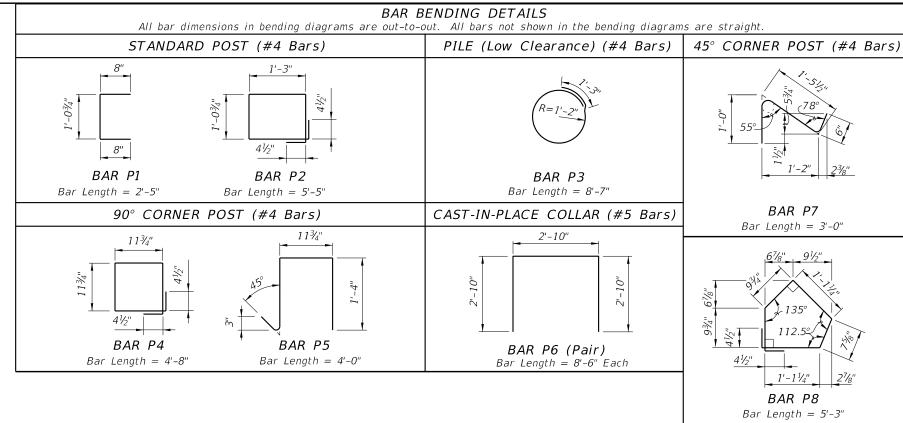
SHEET



* See Sheet 1, Note 4.

VIEW A-A

PILE/POST ELEVATION



		T	ABLE 1	!A - T.	ABLE (	OF PO.	ST RE	INFOR	CING S	STEEL								TAI	BLE 1B	- PILI	E LENG	GTHS (	Feet)	- WIN	D SPE	EED =	130 M	IPH			
	POST LENGTHS WIND SPEED = 130 MPH  10'-0" 20'-0"											10	0'-0" POS	T SPACE	ING					20	"-0" POS	ST SPACI	ING								
NOMINAL WALL	ALL					'-0" SPACING						'-0" SPACING			NOMINAL WALL		H-P	0STS			CORNE	R POSTS			H-P	0STS			CORNEF	R POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	Вя	ARS B	BARS D	В	ARS E	BARS A	BA	RS 3	BARS D	В	ARS E	HEIGHT (Feet)	50	IL 1	50	IL 2	50.	IL 1	501	L 2	501	'L 1	50	IL 2	501	IL 1	50.	IL 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘
12	13'-0½"	13'-2 ¹ / ₂ "	#4	#4	7'-11"	#4	#4	9'-11"	#5	#5	9'-8"	#6	#6	9'-4"	12	11	10	10	10	11	10	10	10	15	14	13	12	14	13	13	12
13	14'-0½"	14'-21/2"	#4	#4	10'-11"	#4	#4	10'-11"	#5	#5	9'-8"	#6	#6	9'-4"	13	12	11	10	10	11	10	10	10	15	14	13	13	15	14	13	12
14	15'-0½"	15'-2 ¹ / ₂ "	#4	#4	10'-11"	#5	#5	11'-8"	#6	#6	11'-4"	#7	#7	10'-8"	14	12	11	11	10	12	11	10	10	16	15	14	13	15	14	14	13
15	16'-0 ¹ / ₂ "	16'-2 ¹ / ₂ "	#4	#4	10'-11"	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8"	15	12	12	11	10	12	11	11	10	16	15	15	13	16	15	14	13
16	17'-0½"	17'-2 ¹ / ₂ "	#5	#5	13'-8"	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8"	16	13	12	11	11	12	12	11	10	17	16	15	14	16	15	15	14
17	18'-0½"	18'-2 ¹ / ₂ "	#5	#5	14'-8"	#5	#5	12'-8"	#7	#7	12'-8"	#7	#8	10'-0"	17	13	12	12	11	13	12	11	11	18	16	16	14	17	16	15	14
18	19'-0 ¹ / ₂ "	19'-2 ¹ / ₂ "	#5	#5	14'-8"	#6	#6	14'-4"	#7	#7	12'-8"	#8	#8	12'-0"	18	14	13	12	11	13	12	12	11	18	17	16	15	18	16	15	14
19	20'-01/2"	20'-21/2"	#5	#5	14'-8"	#6	#6	14'-4"	#7	#8	12'-0"	#8	#9	11'-3"	19	14	13	12	12	14	13	12	11	19	17	16	15	18	17	16	15
20	21'-0½"	21'-2 ¹ / ₂ "	#6	#6	16'-4"	#6	#6	14'-4"	#8	#7	14'-8"	#9	#8	14'-0"	20	14	13	13	12	14	13	12	12	19	18	17	16	19	17	16	15
21	22'-0 ¹ / ₂ "	22'-2 ¹ / ₂ "	#6	#6	16'-4"	#6	#6	14'-4"	#8	#8	14'-0"	#9	#10	12'-4"	21	15	14	13	12	14	13	13	12	20	18	17	16	19	18	17	16
22	23'-01/2"	23'-2 ¹ / ₂ "	#6	#6	16'-4"	#7	#7	16'-8"	#8	#9	13'-3"	#10	#9	15'-3"	22	15	14	14	13	15	14	13	12	20	19	18	17	20	18	17	16

### TABLE NOTE:

- 1. Bars D and Bars E are for 45° Corner Posts only.
- 2. See Contract Plans for project wind speed.
- 3. Soil 1 = Loose Granular Soil, N = 4 to 9.
  - Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

PILE DEPTH & REINFORCING SUMMARY

REVISION 11/01/16

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

NOISE WALLS - (PRECAST)

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		TABLE 2A - TABLE OF POST REINFORCING STEEL  POST LENGTHS WIND SPEED = 150 MPH																TAE	BLE 2B	- PIL	E LEN	GTHS	(Feet)	- WIN	ID SPE	EED =	150 M	IPH			
	POST LI	ENGTHS					WIND	SPEEL	) = 15	O MPH	1							10	0'-0" POS	T SPAC	ING					20	'-0" P05	T SPACI	NG		
NOMINAL WALL						'-0" SPACING						'-0" SPACING			NOMINAL WALL		H-P(	OSTS			CORNE	R POSTS			H-P	0STS			CORNER	POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	B/	NRS B	BARS D	BA L	RS E	BARS A	BA E	RS 3	BARS D	BA	ARS E	HEIGHT (Feet)	501	L 1	50.	IL 2	50.	IL 1	501	L 2	501	IL 1	501	L 2	501	L 1	501	L 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		<i>30</i> " ⊘	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ∅	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ∅	36" ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘
12	13'-01/2"	13'-2 ¹ / ₂ "	#4	#4	9'-11"	#5	#5	9'-8"	#6	#6	9'-4"	#6	#6	8'-4"	12	12	12	11	10	12	11	11	10	17	15	15	14	16	15	14	13
13	14'-0½"	14'-21/2"	#4	#4	9'-11"	#5	#5	10'-8"	#6	#6	9'-4"	#7	#7	8'-8"	13	13	12	11	11	13	12	11	10	17	16	15	14	17	15	15	14
14	15'-0½"	15'-2½"	#5	#5	11'-8"	#5	#5	10'-8"	#7	#7	10'-8"	#7	#7	8'-8"	14	13	12	12	11	13	12	12	11	18	17	16	15	17	16	15	14
15	16'-0 ¹ / ₂ "	16'-2 ¹ / ₂ "	#5	#5	11'-8"	#6	#6	12'-4"	#7	#7	10'-8"	#8	#7	10'-8"	15	14	13	12	11	13	13	12	11	19	17	16	15	18	17	16	15
16	17'-0½"	17'-2½"	#5	#5	11'-8"	#6	#6	12'-4"	#7	#7	10'-8"	#8	#8	10'-0"	16	14	13	13	12	14	13	12	12	19	18	17	16	19	17	16	15
17	18'-0 ¹ / ₂ "	18'-2 ¹ / ₂ "	#6	#6	14'-4"	#6	#6	12'-4"	#7	#8	10'-0"	#9	#8	11'-0"	17	15	14	13	12	14	13	13	12	20	18	17	16	19	18	17	16
18	19'-0½"	19'-2½"	#6	#6	14'-4"	#7	#7	13'-8"	#8	#8	12'-0"	#9	#10	9'-4"	18	15	14	14	13	15	14	13	12	20	19	18	17	20	18	17	16
19	20'-0 ¹ / ₂ "	20'-2 ¹ / ₂ "	#6	#6	14'-4"	#7	#7	13'-8"	#8	#9	11'-3"	#10	#9	12'-3"	19	16	15	14	13	15	14	14	13	21	19	19	17	20	19	18	17
20	21'-0½"	21'-2 ¹ / ₂ "	#6	#6	14'-4"	#7	#8	13'-0"	#9	#9	13'-3"	#10	#10	11'-4"	20	16	15	14	13	16	15	14	13	22	20	19	18	21	19	18	17
21	22'-0½"	22'-2 ¹ / ₂ "	#7	#7	16'-8"	#7	#7	13'-8"	#9	#10	12'-4"	#11	#10	13'-4"	21	17	15	15	14	16	15	14	13	22	21	20	18	21	20	19	18
22	23'-0 ¹ / ₂ "	23'-2 ¹ / ₂ "	#7	#7	16'-8"	#8	#8	16'-0"	#10	#9	14'-3"	#11	#11	12'-5"	22	17	16	15	14	17	15	15	14	23	21	20	19	22	20	19	18

		Τ	ABLE 3	3A - T.	ABLE	OF PO	ST RE	INFOR	CING	STEEL								TAE	BLE 3B	B - PIL	E LEN	GTHS	(Feet)	- WIN	ID SPI	EED =	170 M	1PH			
	POST LENGTHS WIND SPEED = 170 MPH										10	)'-0" POS	ST SPACE	ING					20	'-0" P0S	ST SPACE	ING									
NOMINAL WALL						'-0" SPACING						'-0" SPACING			NOMINAL WALL		H-P(	OSTS			CORNE	R POSTS			H-P(	0STS			CORNEF	R POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	BA	ARS B	BARS D	BA	ARS E	BARS A	BA	IRS B	BARS D	Вя	ARS E	HEIGHT (Feet)	501	L 1	50.	IL 2	50	IL 1	501	L 2	501	'L 1	501	L 2	50.	OIL 1	50	IL 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		<i>30</i> " ⊘	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ∅	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ∅	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ∅	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘
12	13'-0 ¹ / ₂ "	13'-2 ¹ / ₂ "	#5	#5	9'-8"	#5	#5	8'-8"	#6	#6	8'-4"	#7	#7	7'-8"	12	14	13	12	11	13	12	12	11	18	17	16	15	18	16	16	15
13	14'-0½"	14'-2 ¹ / ₂ "	#5	#5	10'-8"	#6	#6	10'-4"	#7	#7	8'-8"	#8	#7	8'-8"	13	14	13	13	12	14	13	12	11	19	18	17	16	19	17	16	15
14	15'-0 ¹ / ₂ "	15'-2 ¹ / ₂ "	#5	#5	10'-8"	#6	#6	10'-4"	#7	#7	8'-8"	#8	#8	8'-0"	14	15	14	13	12	14	13	13	12	20	18	18	16	19	18	17	16
15	16'-0 ¹ / ₂ "	16'-2 ¹ / ₂ "	#6	#6	12'-4"	#6	#6	10'-4"	#8	#7	10'-8"	#9	#8	10'-0"	15	15	14	14	13	15	14	13	12	21	19	18	17	20	18	18	16
16	17'-0½"	17'-2½"	#6	#6	12'-4"	#7	#7	11'-8"	#8	#8	10'-0"	#9	#10	8'-4"	16	16	15	14	13	15	14	14	13	21	20	19	17	21	19	18	17
17	18'-0 ¹ / ₂ "	18'-2 ¹ / ₂ "	#6	#6	12'-4"	#7	#7	11'-8"	#9	#8	12'-0"	#10	#9	10'-3"	17	16	15	15	14	16	15	14	13	22	20	19	18	21	20	19	17
18	19'-0 ¹ / ₂ ''	19'-2 ¹ / ₂ ''	#7	#7	13'-8"	#7	#8	11'-0"	#9	#10	10'-4"	#10	#11	8'-5"	18	17	16	15	14	16	15	15	14	23	21	20	19	22	20	19	18
19	20'-0 ¹ / ₂ "	20'-2 ¹ / ₂ "	#7	#7	13'-8"	#8	#7	13'-8"	#10	#10	11'-4"	#11	#11	10'-5"	19	17	16	15	14	17	16	15	14	23	22	21	19	23	21	20	18
20	21'-0 ¹ / ₂ "	21'-2 ¹ / ₂ "	#7	#7	13'-8"	#8	#8	13'-0"	#10	#11	10'-5"	#11	#14	7'-0"	20	18	17	16	15	17	16	15	14	24	22	21	20	23	21	20	19
21	22'-0 ¹ / ₂ "	22'-2 ¹ / ₂ "	#7	#8	13'-0"	#9	#8	15'-0"	#11	#10	13'-4"	#14	#11	12'-5"	21	18	17	16	15	18	17	16	15	25	23	22	20	24	22	21	19
22	23'-0 ¹ / ₂ "	23'-2 ¹ / ₂ "	#8	#7	16'-8"	#9	#9	14'-3"	#11	#11	12'-5"	#14	#14	9'-0"	22	19	18	17	16	18	17	16	15	25	23	22	21	24	23	22	20

#### TABLE NOTE:

- 1. Bars D and Bars E are for 45° Corner Posts only.
- 2. See Contract Plans for project wind speed.
- 3. Soil 1 = Loose Granular Soil, N = 4 to 9;

≥ DESCRIPTION:

Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

PILE DEPTH & REINFORCING SUMMARY

REVISION 11/01/16

FDOT

FY 2019-20 STANDARD PLANS

NOISE WALLS - (PRECAST)

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#### GENERAL NOTES:

- 1. Construct Perimeter Walls in accordance with Specification Section 534.
- 2. Choice of either Precast Option or Masonry Option is at the discretion of the Contractor. Contractor must also select the desired foundation type. Modifications to this Index is restricted to those required for geometric needs only.
- 3. Post spacing is measured from centerline to centerline of foundation element. For this Index, posts and foundation elements have been designed for 20 ft. spacings. Use post spacings less than 20 feet only at changes in horizontal alignment, wall terminations or to accommodate steep grades.
- 4. See "Perimeter Wall Data Tables" in the plans for project requirements.
- 5. Field verify the locations of all overhead and underground utilities shown in the Wall Control Drawings.

#### PRECAST OPTION NOTES:

- 6. WALL NOTES:
- A. Walls may consist of either a single height panel or two stacked panels. Minimum panel height is 4'-3".
- B. Only when reduced overhead clearance between posts prohibits installation of panels from the top, side-installed panels are allowed. After panel is centered between posts, grout between panel ends and posts.
- 7. CONCRETE AND GROUT:
- A. Cast-in-Place and Precast Concrete: Class IV
- B. Grout for Auger Cast Piling: Minimum 28 Day Strength = 5000 psi
- C. Minimum Compressive Strength for Form Removal and Handling of Posts, Panels and Precast Spread Footings:
  - i. 2,500 psi for horizontally cast post, panels and precast spread footings.
  - 2,000 psi for vertically cast panels or when tilt-up form tables are used for horizontally cast panels.
- 8. REINFORCING STEEL:
- A. Concrete Cover:  $1\frac{1}{2}$ " unless otherwise noted.
- B. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
- i. Post Stirrups Tie at all four corner bars and at every third interior bar intersection.
- ii. Pile Stirrups Tie to the main vertical reinforcing at alternate intersections.
- 9. BEARING PADS
- A. Bearing Pads for Collar or Pedestal Bearing Points and between stacked panels may be either Plain or Fiber Reinforced Neoprene Pads, in accordance with Specification Section 932 for ancillary structures.
- 10. CASTING TOLERANCES:
- A. Overall Height & Width:  $+/-\frac{1}{4}$ "
- B. Thickness: +/- 1/4"
- C. Plane of side mold:  $\pm -\frac{1}{16}$ "
- D. Openings:  $+/-\frac{1}{2}$ "
- E. Out of Square:  $\frac{1}{8}$  per 6 ft., but not more than  $\frac{3}{8}$  total along any side
- F. Warping:  $\frac{1}{16}$ " per foot distance to nearest corner
- G. Bowing: 1/240 panel dimension
- 11. PILING

Construct Auger Cast Piling in accordance with the Plans and Specification Section 455.

#### MASONRY OPTION NOTES:

DESCRIPTION:

- 12. WALL NOTES:
- A. Inspect construction in accordance with the International Building Code (IBC) Section 17.
- B. Construct masonry walls with 8x8x16 block using a running bond pattern and concave tooled joints.
- C. Make all elevation changes (steps) in footing and top of wall using full height blocks. Make top of wall steps at pilasters exclusively. Footing steps may be made between pilasters as necessary to maintain minimum soil cover.

#### MASONRY OPTION NOTES (CONT.):

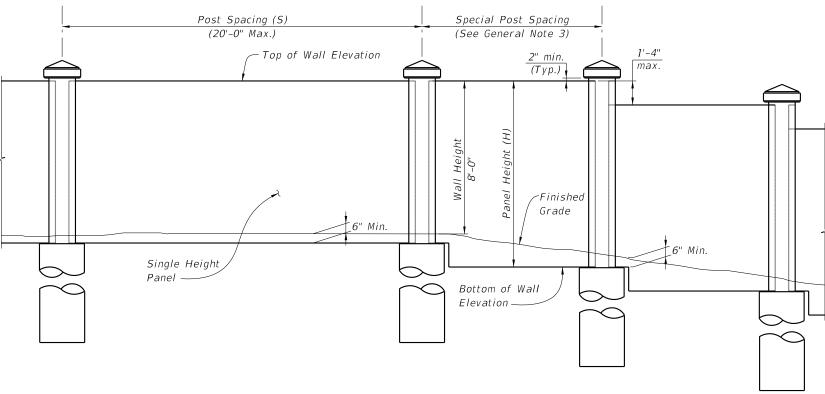
- D. Fully Grout all cells with horizontal or vertical reinforcing bars.
- E. Use reinforcing bar positioners to maintain vertical and horizontal bar placement.
- F. Fully grout first three courses of the wall.
- G. Joint Reinforcement: Use W 1.7 (9mm) galvanized ladder reinforcing spaced at 16" vertically. Provide special accessories for corners, intersections, etc. Joint reinforcing shall be continuous except it shall not pass through vertical masonry control joints. Lap joint reinforcing a minimum of 6".
- H. Construct expansion joints in the foundation at 90 foot maximum intervals, and directly below a wall control joint.
- I. Dowel Load Transfer Devices will be ASTM A 36 smooth round bars hot-dip galvanized in accordance with Specification Section 962. Install Dowel Load Transfer Devices in accordance with Specification Section 350.
- J. For spread footings, use a walk-behind compactor of at least 600 lbs. in weight. Obtain a minimum density of 95% of the maximum dry density as determined by FM 1 T-180. Perform soil density tests at 100 foot intervals.
- K. Protect walls during construction from soil, grout or mortar stains. Clean wall as work progresses by dry brushing to remove mortar fins and smears before tooling joints.
- L. Use soap and potable water to clean walls. If stain removal is necessary, use a cleaning method indicated in NCMA TEK 8-2A applicable to the type of stain on the exposed surface.
- M. During construction, cover tops of walls, with waterproof sheeting at the end of each day's work, or when construction is not in progress. Extend sheeting a minimum of 2 feet down each side and secure in place.
- N. Comply with Hot Weather Requirements in ACI 530.1.

#### 13. MATERIALS:

- A. Concrete Masonry Units (CMU): Provide normal weight blocks.
- B. Cast-In-Place Concrete: Class II for slightly to moderate aggressive environments or Class IV for extremely aggressive environments
- C. Mortar: Type S meeting requirements of ASTM C1329
- D. Grout: Type S; coarse grout.
- E. Aggregate for Grout: Meet the requirements of ASTM C404 or Specification Section 901 size 8 or 89.

#### 14. STORAGE OF MATERIALS:

- A. Store CMU's on elevated platforms in a dry location or under cover.
- If units become wet, do not install until they are dry.
- B. Store cementitious materials on elevated platforms, under cover, and in a dry location. Do not use cementitious materials that have become damp or exceeded the manufacturers shelf life.
- C. Store masonry accessories and reinforcing to prevent corrosion and accumulation of dirt and oil.



GENERAL WALL ELEVATION
(Precast Option with SIngle Height Panel Shown, Others Similar)

GENERAL NOTES

LAST REVISION 11/01/17



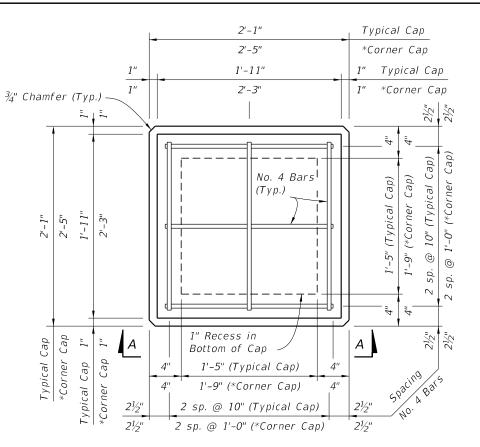
FY 2019-20 STANDARD PLANS

PERIMETER WALLS

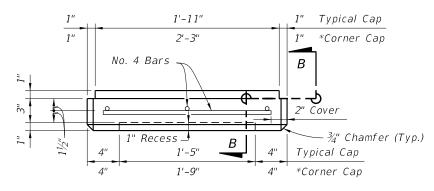
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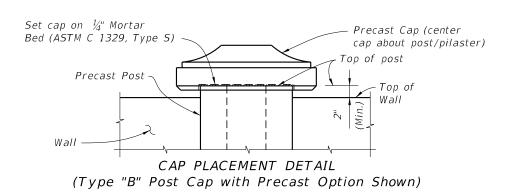
10/30/2018

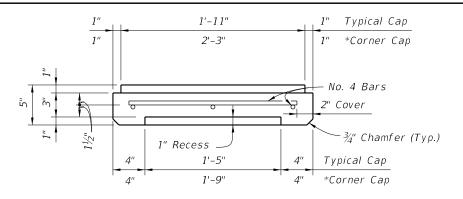


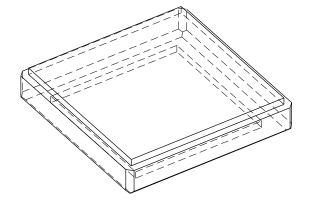
PLAN VIEW (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



VIEW A-A (Type "A" Cap Shown, Type "B" & "C" Caps Similar)





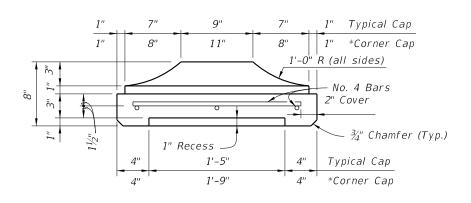


SECTION B-B

PICTORIAL VIEW

= TYPE "A" CAP DETAILS =

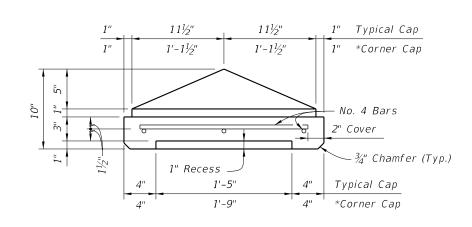
*Precast Option only

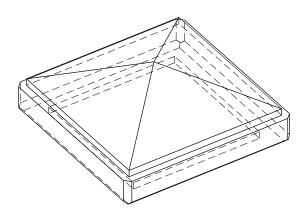


SECTION B-B

PICTORIAL VIEW

TYPE "B" CAP DETAILS





SECTION B-B

PICTORIAL VIEW

= TYPE "C" CAP DETAILS =

POST CAP DETAILS

REVISION 01/01/14

DESCRIPTION:

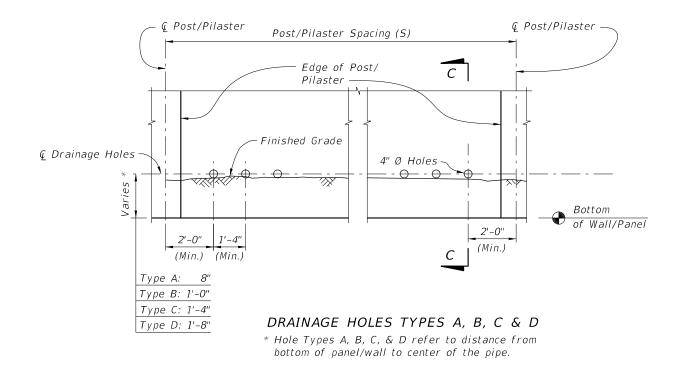
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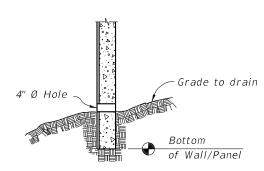
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PERIMETER WALLS

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SECTION C-C (Precast Option Shown, Masonry Option Similar)

- 1. Drainage holes may be formed with 4" NPS PVC pipe that may remain in place.
- 2. See Wall Control drawings for number, Type and location/ spacing of drainage holes.

DRAINAGE DETAILS

REVISION 01/01/14

FDOT

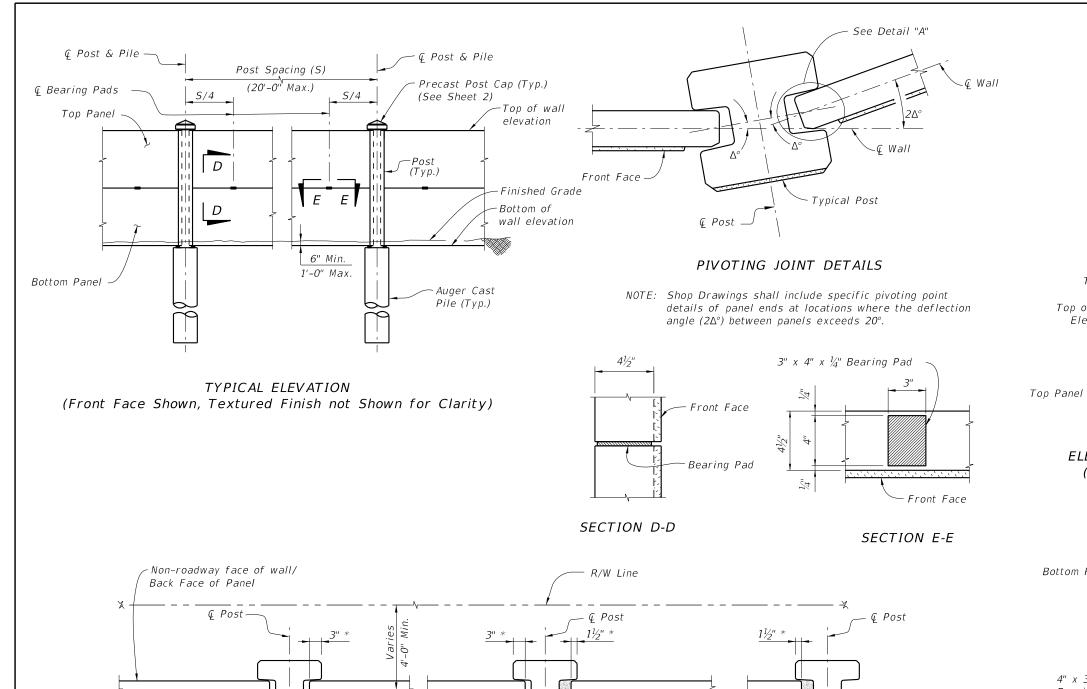
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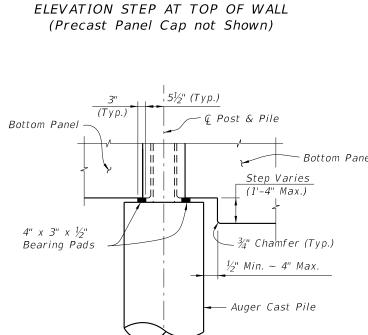
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≥ DESCRIPTION:





Required

Precast Cap

(Type "C" Shown)

Step Varies

-Top Panel

(1'-4" Max.)

DETAIL "A"

(Back Face Chamfer Shown

Front Face Chamfer Similar)

Top of Post

Top of Wall

. Elevation

Bottom Panel

ELEVATION STEP AT BOTTOM OF WALL

PRECAST OPTION - TYPICAL DETAILS

**REVISION** 11/01/17

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L (Top-Installed)

TYPICAL PLAN

FY 2019-20 STANDARD PLANS

Fill with Non-Shrink Grout

L (Side-Installed)

* Nominal embedment (not including tolerances)

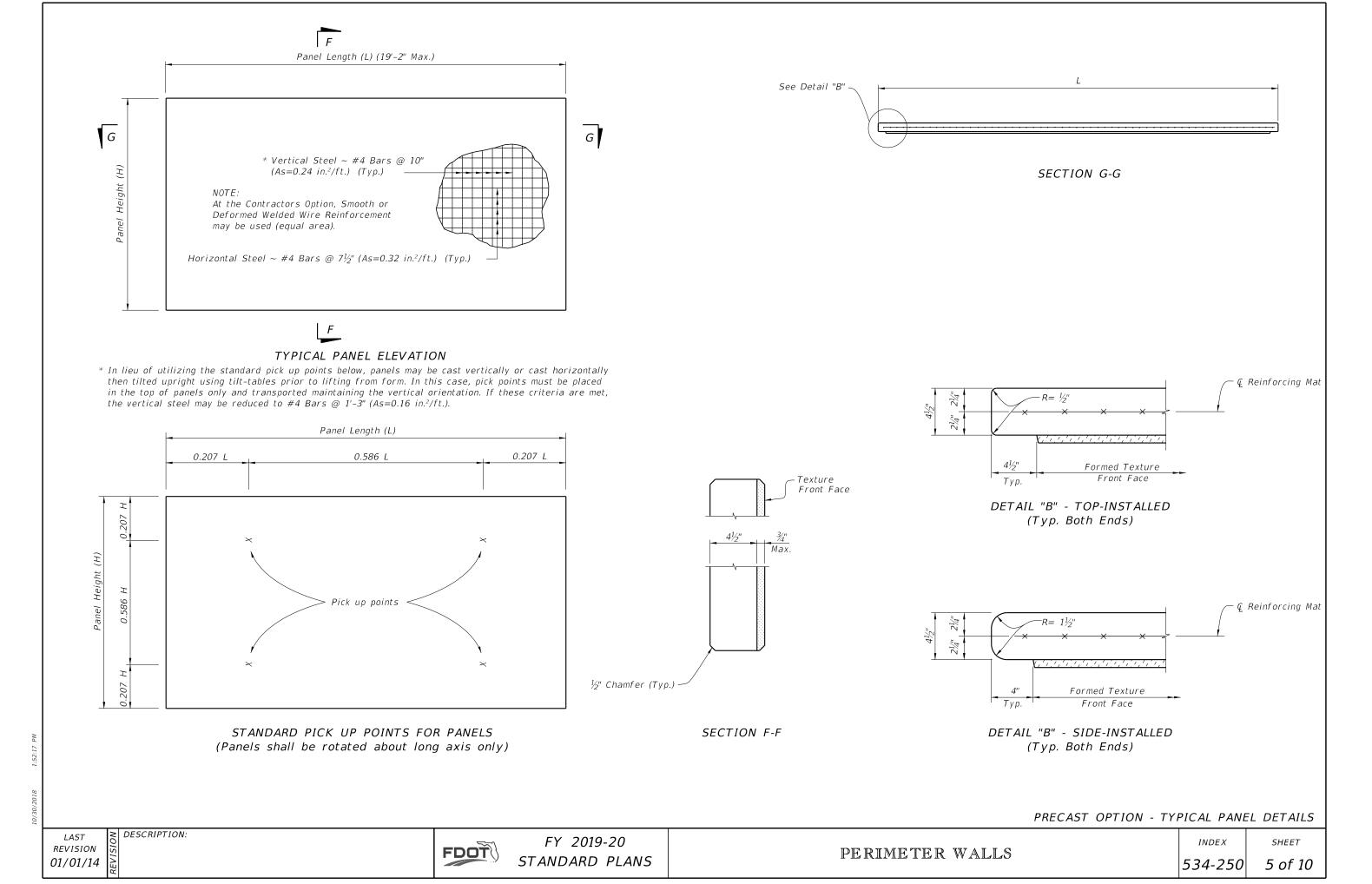
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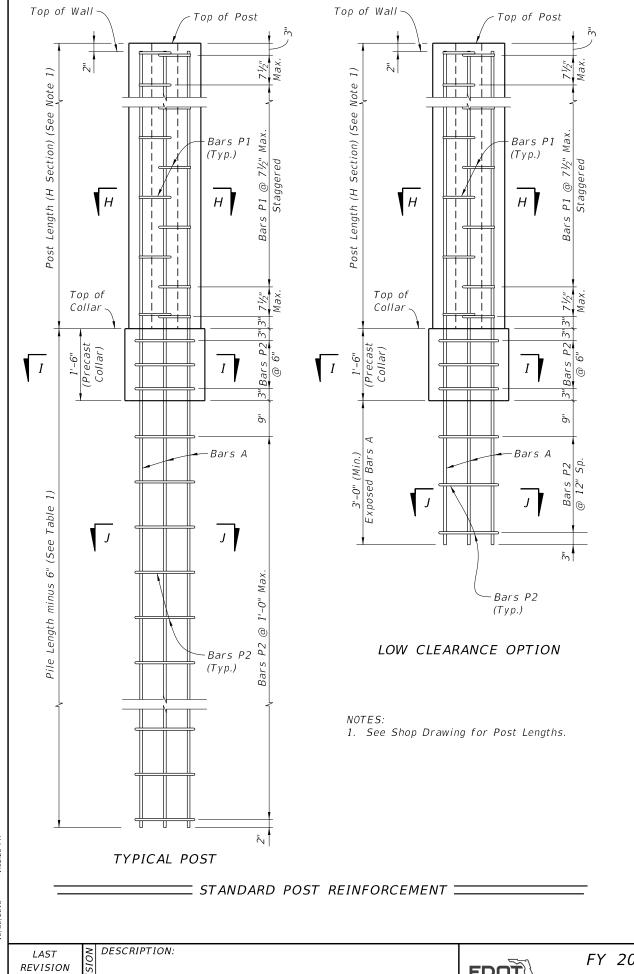
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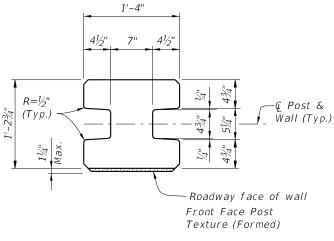
DESCRIPTION:

Roadway face of wall/ Front Face of Panel -

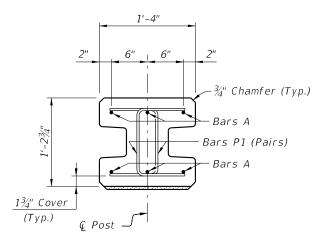




11/01/17

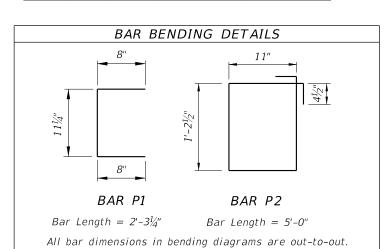


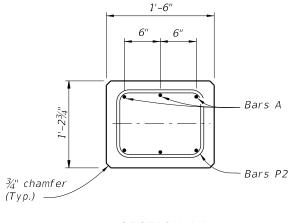
## TYPICAL POST SECTION (H Section)



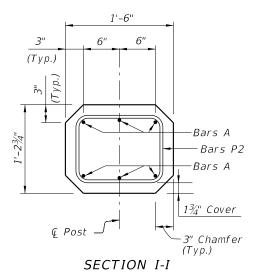
SECTION H-H (H Section - Above Collar)

		T	ABLE 1	
Wind Speed (MPH)	Pile Length	Bars A	Bars P1 thru P6	Bars S1
130	12'-0"	#5	#3	#4
150	13'-6"	#5	#3	#4
170	15'-0"	#6	#3	#4

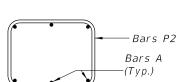




SECTION I-I Precast Collar



(for Low Clearance Option)



SECTION J-J

PRECAST OPTION - STANDARD POST DETAILS

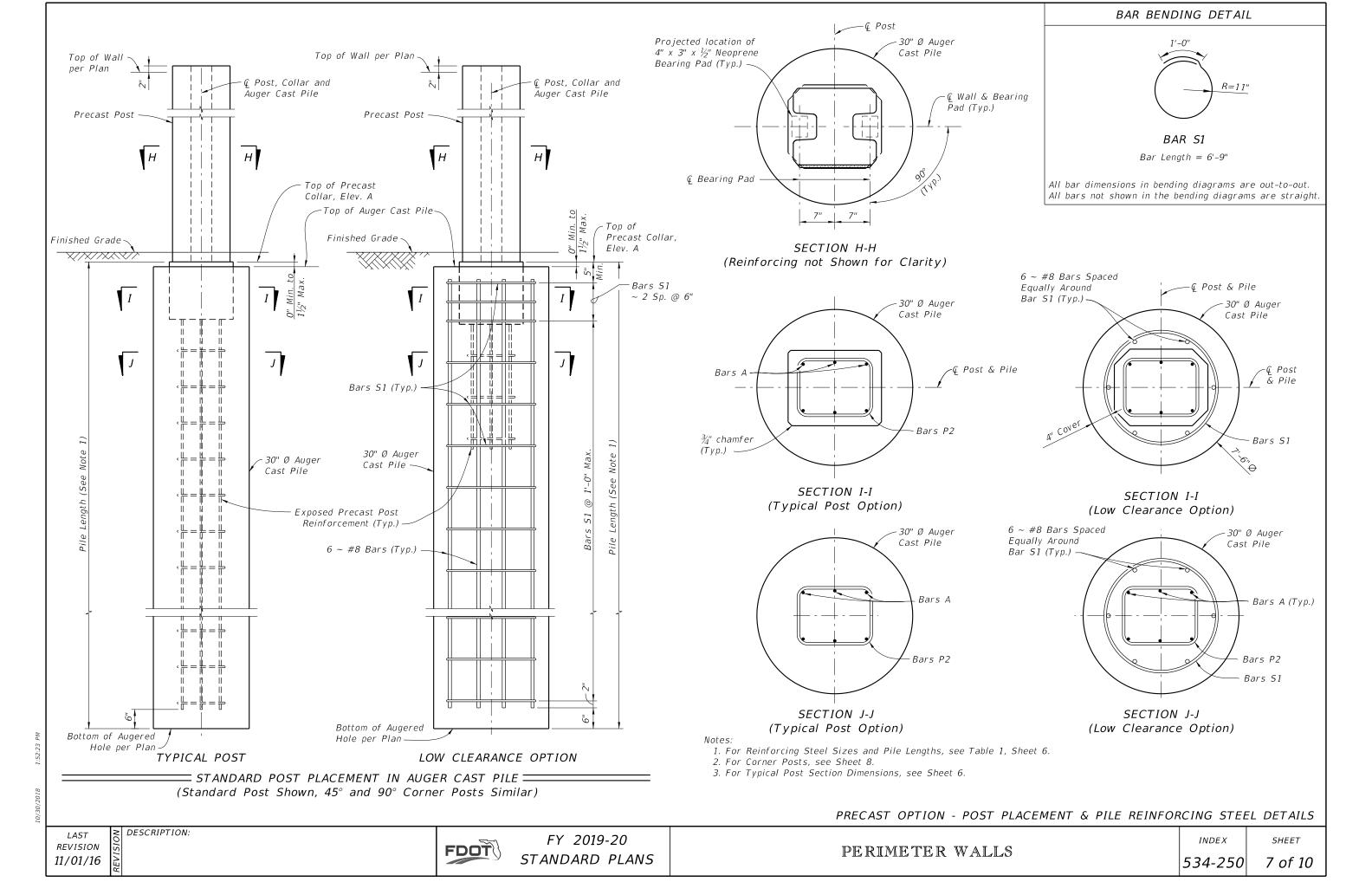
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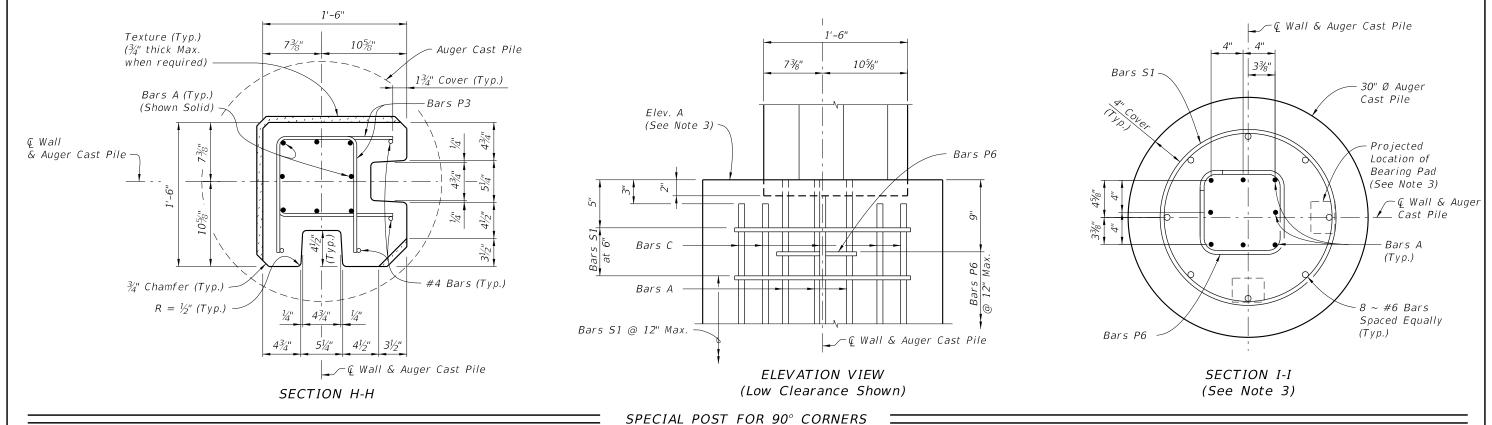
FY 2019-20 STANDARD PLANS All bars not shown in the bending diagrams are straight.

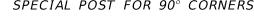
PERIMETER WALLS

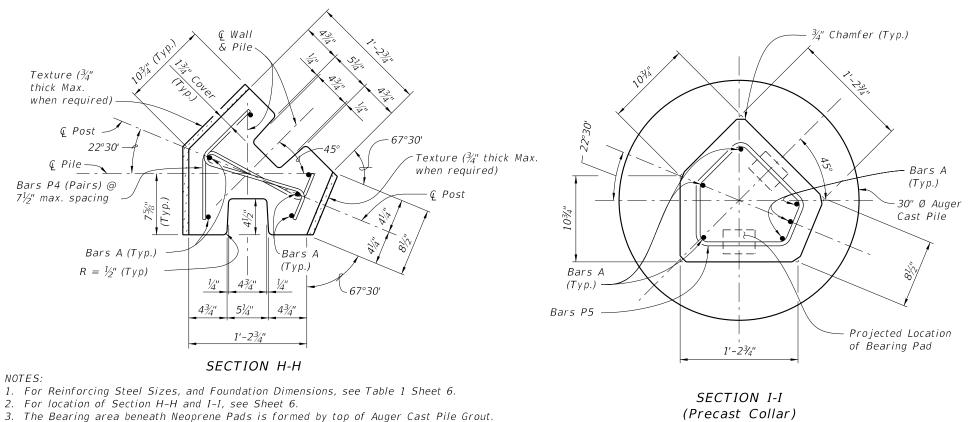
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BAR BENDING DETAILS 10½ 10½" BAR P3 BAR P6 (90° Corner) (90° Corner) Bar Length =  $3' - 3^{1}/3''$ Bar Length = 4'-2'' $1'-2\frac{1}{2}'$ BAR P4 BAR P5 (45° Corner) (45° Corner) Bar Length =  $2'-3\frac{1}{2}''$ Bar Length =  $4'-6^{1/3}$ "

All bar dimensions in bending diagrams are out-to-out. All bars not shown in the bending diagrams are straight.

## SPECIAL POSTS FOR 45° CORNERS

PRECAST OPTION - SPECIAL CORNER POSTS

**REVISION** 11/01/17

DESCRIPTION:

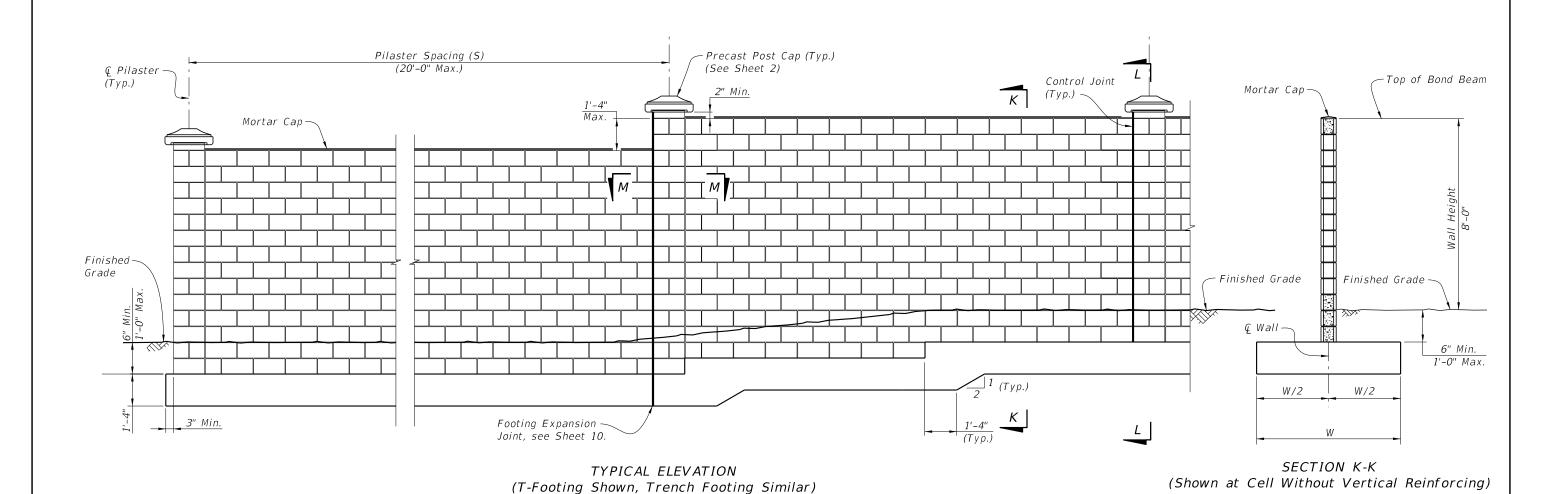
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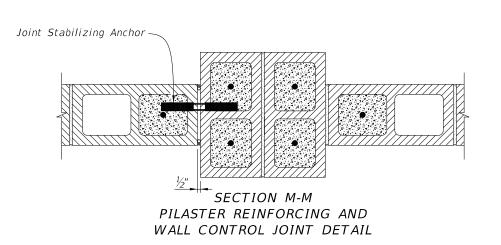
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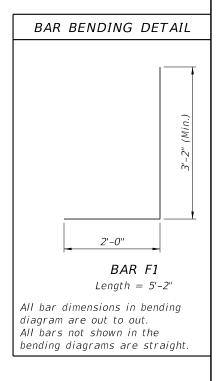


		Tab	ole 2		
Wind	Masonry	v Walls		Foundatior	ns
Speed Category	(8x8)	x16)	Bars	T-Footing Width	Trench Footing
l caregory	Bars V1	SV Spacing	F1 & F2	(W)	Depth (D)
130	#5	2'-8"	#5	4'-4"	5'-6"
150	#5	2'-0"	#5	5'-0"	6'-4"
170	#5	1'-4"	#5	6'-0"	7'-0"

- 1. End vertical reinforcing bars  $1\frac{1}{2}$ " from top of bond beam blocks and horizontal bars  $1\frac{1}{2}$ " from edge of control joints.
- 2. Do not continue horizontal #4 Bond beam reinforcing through control joint.
- 3. Use stainless steel joint stabilizing anchors spaced at 16" vertically at all control joints. Install per manufacturers instructions.
- 4. Seal Control Joints with backer rod and Type "A" silicone sealant (top and both sides).
- 5. See Sheet 10 for Bar placement details.
- 6. For Pilaster Cap Details, see Sheet 2.

DESCRIPTION:





**MASONRY OPTION** 

**REVISION** 11/01/17

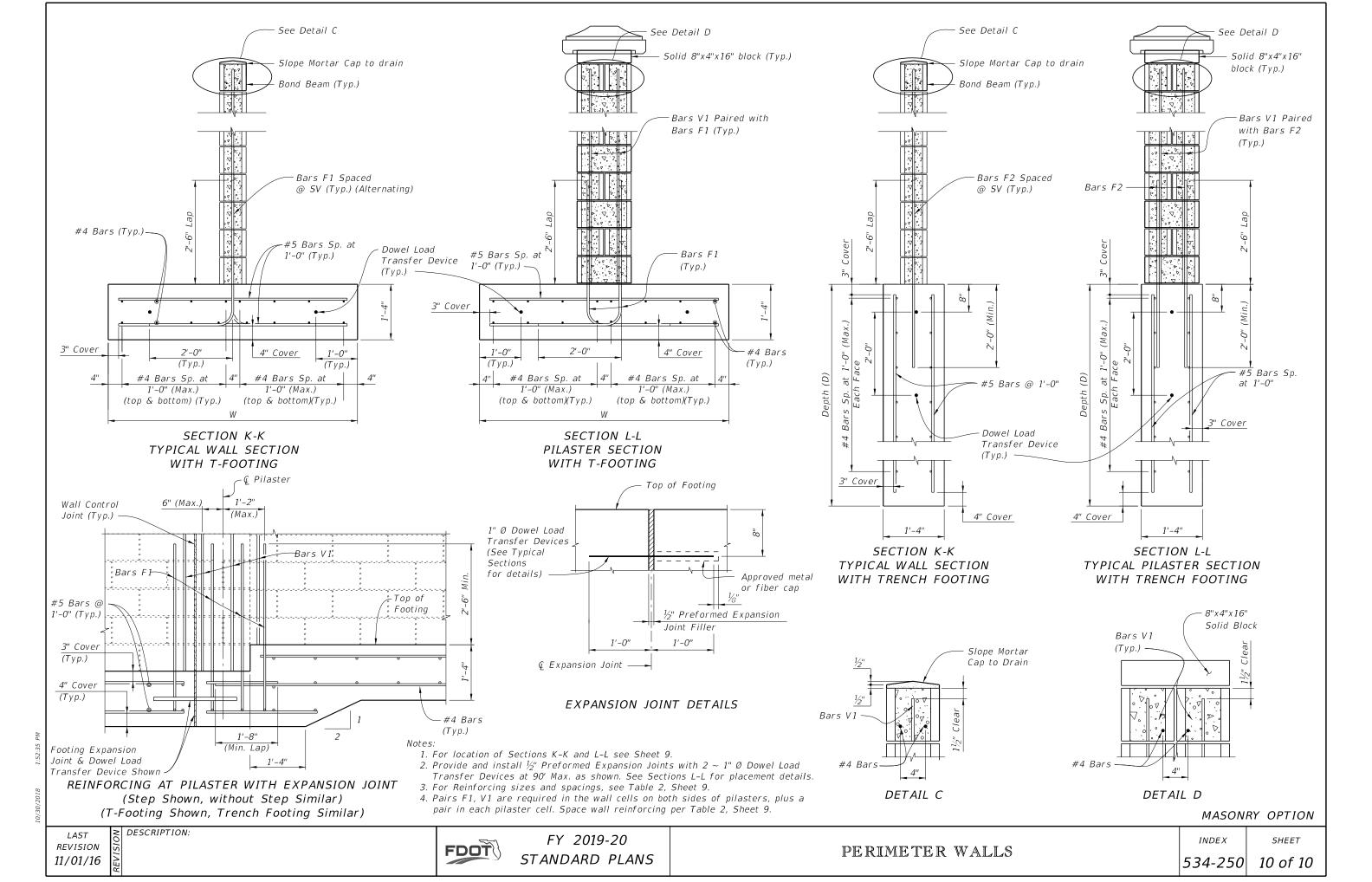
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PERIMETER WALLS

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Modified Mount - Frangible Leave-Out for Concrete Surface Mount  Barrier Delineators - Post Mounted;  Clear Space - Reduced Post Spacing for Hazards;		Modified Mount - Special Steel Post for Concrete Structure Mount;
Barrier Delineators - Post Mounted;  22 Clear Space - Reduced Post Spacing for Hazards;	21	Modified Mount - Encased Post for Shallow Mount;
22 Clear Space - Reduced Post Spacing for Hazards;		Modified Mount - Frangible Leave-Out for Concrete Surface Mount
		Barrier Delineators - Post Mounted;
5%" Button-Head Bolt System	22	Clear Space - Reduced Post Spacing for Hazards;
		%" Button-Head Bolt System

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LAST REVISION 11/01/18

DESCRIPTION:

CUEET CONTENTS



FY 2019-20 STANDARD PLANS

#### GENERAL NOTES:

1. INSTALLATION: Construct guardrail in accordance with Specification 536.

This Index, along with the plans and the manufacturers' drawings on the Approved Products List (APL), is sufficiently detailed for installation of General Guardrail, Low-Speed Guardrail, End Treatment assemblies, and their connecting options shown herein. This precludes requirements for shop drawing submittals unless otherwise specified in the plans.

- 2. COMPATIBILITY: The General Guardrail in this Index is based on the Midwest Guardrail System (MGS) design, with an approximate height of 31" at the top of the Panel (2'-1" mounting height at vertical Q of Panel) and a midspan panel splice as shown on Sheet 2. Guardrail components included on the APL, which are compatible with this Index, may also be identified as 31" or MGS Guardrail.
- 3. STANDARD COMPONENTS: Standard guardrail components, including posts, panels, and bolt systems, are based upon English unit conversions of the AASHTO-AGC-ARTBA Joint Committee Task Force 13 Report: A Guide to Standardized Highway Barrier Hardware (http://www.aashtotf13.org/Barrier-Hardware.php).
- 4. BUTTON-HEAD BOLTS: Install Button-Head Bolts where indicated using bolts, nuts, and washers as defined on Sheet 22. Place washers under nuts. Do not place washers between bolt heads and panels, except where otherwise shown in this Index.
- 5. HEX-HEAD BOLTS: Install Hex-Head Bolts where indicated using bolts, nuts, and washers in accordance with material properties of Specification 967. Place washers under nuts.
- 6. MISCELLANEOUS ASPHALT PAVEMENT: Install Miscellaneous Asphalt Pavement where indicated with a tolerance of  $\pm \frac{1}{2}$ " depth and in accordance with Specification 339.
- 7. ADJACENT SIDEWALKS & SHARED USE PATHS: When guardrail posts are placed within 4'-0" of a sidewalk or shared use path, use timber posts, or use steel posts only if treated with Pipe Rail as shown on Sheet 20.

When timber posts are used, one of the following safety treatments is required for the bolt(s) protruding from the back face of the posts:

- a. After tightening the nut, trim the protruding post bolt flush with the nut and galvanize per Specification 562.
- b. Use post bolts 15" in length and countersink the washer and nut between 1" and 11/3" deep into the back face of the post.
- c. Use 15" post bolts with sleeve nuts and washers.

When End Treatment posts are within 4'-0" of a sidewalk or shared use path, steel posts are not permitted within the End Treatment segment. Terminate the Pipe Rail outside of End Treatment segments, as noted per Sheet 20.

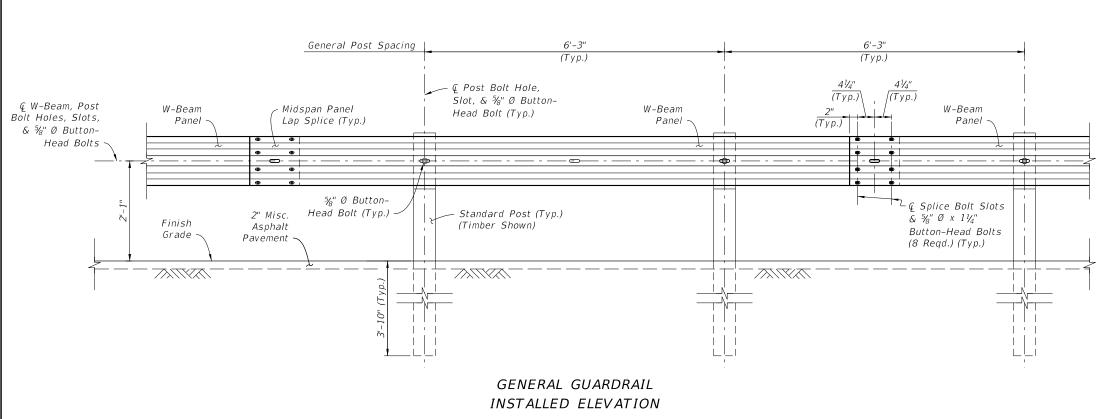
- 8. NESTED W-BEAM: Where called for in the plans, install two W-Beam Panels mounted flush per location, securing all panels with Button-Head Bolts threaded through aligned slots and holes. 2" Button-Head Bolts are permitted for panel splice locations.
- 9. CONNECTION TO RIGID BARRIER: The connections to Rigid Barrier in this Index only apply to newly constructed bridge Traffic Railing and Concrete Barrier or where the complete Approach Transition Connection to Rigid Barrier shown herein can be installed without conflicting with existing Traffic Railings, structures, or approach slabs.

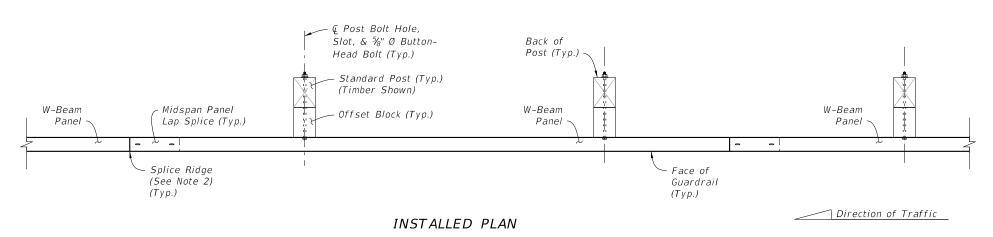
For connecting guardrail to existing bridge Traffic Railings, see the layouts and details of Indexes 536-002, 521-404, and 421-405.

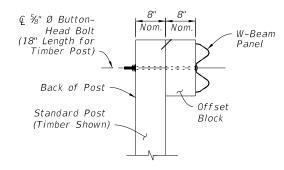
- 10. CONNECTION TO EXISTING GUARDRAIL: Where a transition to existing guardrail at 27" height is required, linearly transition the guardrail height over a distance ranging from 25'-0" to 31"-3". Provide an immediate transition to the required midspan splice using the available panel options on Sheet 4 (9'-4\%" or 15'-7\%" panel).
- 11. PLANS CALLOUTS: Begin/End Station labels are shown throughout this Index as they correspond to the station and offset callouts specified in the plans.

In the plans, Begin/End Guardrail Station refers to the General TL-3 Guardrail Pay Item, and it may be abbreviated as Begin/End GR. Station. Where the Low-Speed TL-2 Guardrail Pay Item is specifically required, the callout in the plans will then specify Begin/End TL-2 GR. Station.

12. QUANTITY MEASUREMENT: Measure guardrail and corresponding components as defined in Specification 536. The Guardrail length is measured along the centerline of installed Panels, between the points labeled Begin/End Guardrail Station shown on the following Index Sheets and defined in the plans (typically measured from the <code>Q</code> of the panel's post bolt slots at the approach/trailing ends).







#### INSTALLED SECTION

#### NOTES:

1. GENERAL: Install the General Guardrail configuration where indicated in the plans. This may include tapered segments if called for in the plans.

Use 12'-6" or longer W-Beam Panels. A single 6'-3" Panel may be used at the end of the run to meet the nominal Begin/End Guardrail Sta. requirements.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 21 for additional post mounting options.
- 8. DEFINED SEGMENTS: The General Guardrail shown provides the base configuration, including Post Spacing and splice locations, for Defined Segment modifications where indicated in the plans and using the Guardrail Types, Sections, and/or hardware as shown in this Index (e.g. Double Faced W-Beam, Modified Thrie-Beam, Deep Posts at Slope Breaks, Pipe Rail, Rub Rail, or Reduced Post Spacing for Hazards).

GENERAL, TL-3 GUARDRAIL DETAILS

**REVISION** 11/01/17

DESCRIPTION:

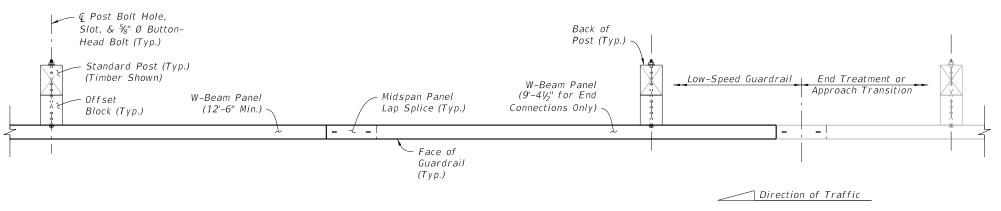
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FY 2019-20 STANDARD PLANS

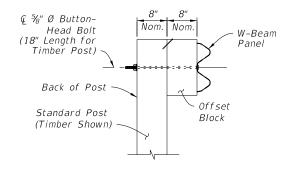
INDEX 536-001

SHEET

GUARDRAIL



INSTALLED PLAN



#### INSTALLED SECTION

#### NOTES:

1. GENERAL: Install the Low-Speed Guardrail configuration where indicated in the plans. Low-Speed Guardrail may include tapered segments if called for in the plans.

Use 12'-6" or 25'-0" W-Beam Panels for normal spans, and use  $9'-4\frac{1}{2}"$  Panels for end connections to adjoining segments as shown. A single 6'-3" Panel may be used at the end of the Low-Speed Guardrail run along with a single reduced 6'-3" post spacing to meet the nominal Begin/End Guardrail Sta. required.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the Plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 21 for additional post mounting options.
- 8. RESTRICTIONS: Low-Speed Guardrail segments are not permitted for use with items including, but not limited to, Double Faced W-Beam, Modified Thrie-Beam, Deep Posts at Slope Breaks, Pipe Rail, and/or Rub Rail.

## LOW-SPEED, TL-2 GUARDRAIL DETAILS

**REVISION** 11/01/17

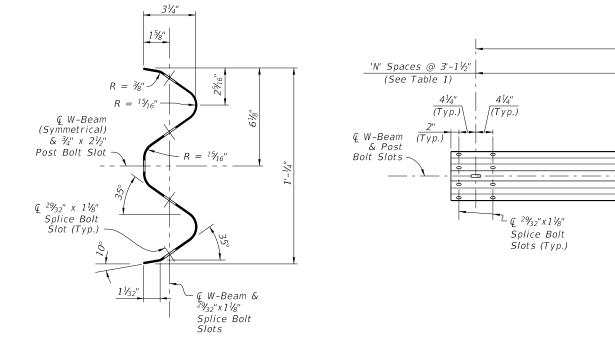
DESCRIPTION:

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FY 2019-20 STANDARD PLANS

INDEX 536-001 SHEET

GUARDRAIL



W-BEAM PANEL SECTION

 $R = {}^{15}/_{16}$ "

(Typ.)

€ W-Beam &

²9⁄₃₂" x 1 ½" Splice Bolt Slots

THRIE-BEAM PANEL SECTION

31/4"

15/8"

 $R = {}^{15}\!/_{16}$ " (Typ.)

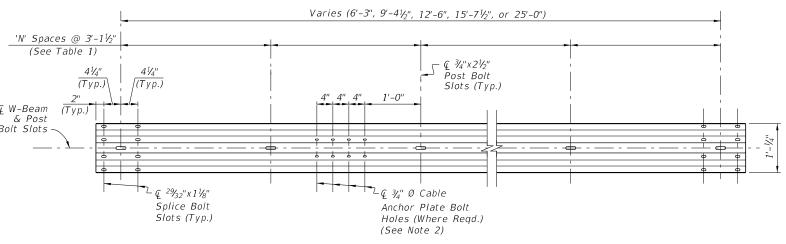
Q ¾" x 2½" Post Bolt Slot

> € Thrie-Beam (Symmetrical)

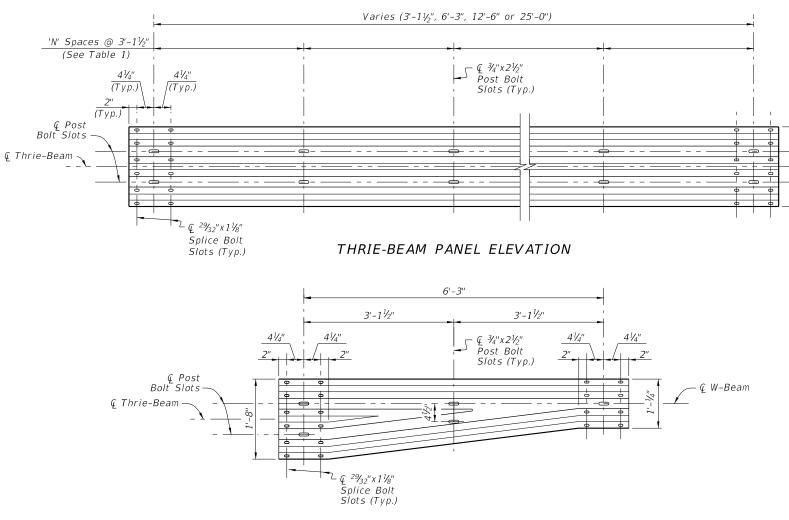
Q ²⁹/₃₂" x 1½"

Splice Bolt

Ślot (Typ.)



# W-BEAM PANEL ELEVATION



## THRIE-BEAM TRANSITION PANEL ELEVATION (Reverse Direction Similar by Opposite Hand)

#### PANEL SUMMARY TABLE:

Panel Type	Number of Spaces 'N'	Gauge
6'-3" W-Beam	2	12
9'-4½" W-Beam	3	12
12'-6" W-Beam	4	12
15'-7½" W-Beam	5	12
25'-0" W-Beam	8	12
3'-1½" Thrie-Beam	1	10
6'-3" Thrie-Beam	2	12
12-6" Thrie-Beam	4	12
25-0" Thrie-Beam	8	12
Thrie-Beam Trans.	2	10

### NOTES:

1. MATERIALS:

Use corrugated steel panels in accordance with Specification 967 and made from either Class A, 12 gauge steel or Class B, 10 gauge steel as specified in the 'Panel Summary Table' above.

2. CABLE ANCHOR PLATE BOLT HOLES: Include 3/4" Ø Cable Anchor Plate Bolt Holes only where required for installation of the Cable Anchor Plate shown on Sheet 9, 10, & 11.

 $^{2}\%_{32}$ " x 1%" slots may substitute for the  $^{3}4$ " Ø holes shown.

> W-BEAM AND THRIE-BEAM PANEL DETAILS

LAST **REVISION** 11/01/17

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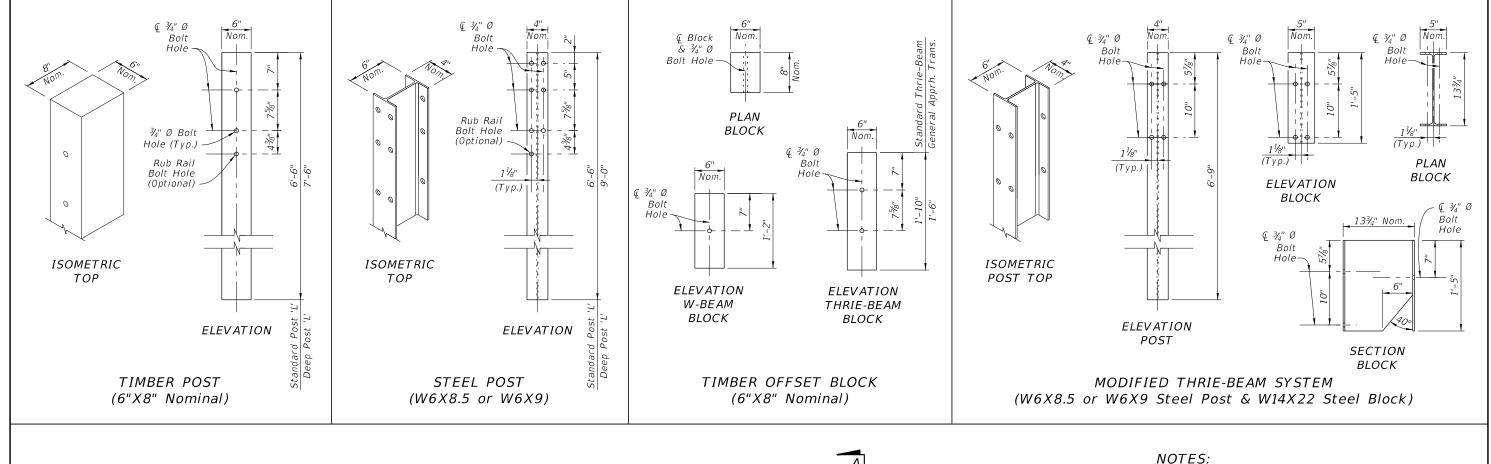
FY 2019-20 STANDARD PLANS

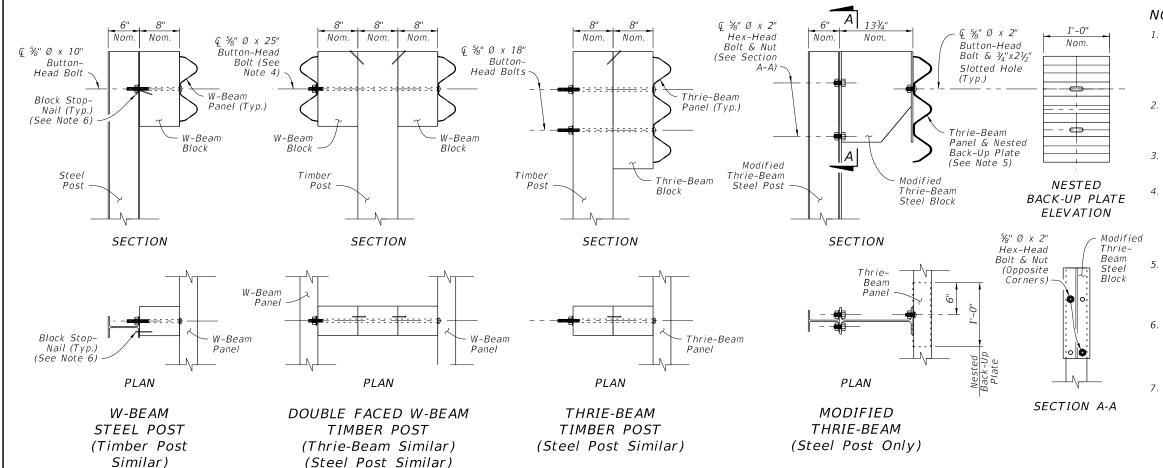
GUARDRAIL

INDEX 536-001

SHEET 4 of 22

DESCRIPTION:





- 1. STANDARD POSTS: Where Standard Posts are called for in this Index, use either a Timber Post or Steel Post at the Length, 'L', shown for Standard Posts. Use a single post material type consistently per each run of guardrail. Only where specified in the Plans, use the Deep Post 'L' for Slope Break Conditions as shown on Sheet 6.
- 2. OFFSET BLOCKS: For each Panel type, install the corresponding Offset Block type as shown. For General, TL-3 (Single Faced) Approach Transitions only, use the 1'-6" Thrie-Beam Block (See Sheet 13).
- 3. BOLT HOLES: 3/1 Ø Bolt Holes shown in posts within this Index may be substituted with  ${}^{13}\!\!/_{16}$ " Ø Bolt Holes.
- 4. DOUBLE FACED GUARDRAIL: Orient Post Bolts with the Button-Head located on the side nearest the traffic lane. The bolt's threaded portion is not permitted to extend beyond 3/4" from the face of the tightened nut; trim the threaded portion as needed and galvanize in accordance with Specification 562.
- 5. MODIFIED THRIE-BEAM NESTED BACK-UP PLATE: At each post connection, install a Nested Back-up Plate between the Thrie-Beam Panel and the post. The Nested Back-up Plate has a cross-section and material matching the Thrie-Beam Panel Section.
- 6. BLOCK STOP-NAIL: Drive one nail per Standard Offset Block as shown to prevent Block rotation. Use steel 31/2" Type 16d nails with ASTM A153 hot-dip galvanization. For steel posts, drive the nail through the unused flange Bolt Hole and bend the nail so its head contacts the flange.
- 7. MATERIALS: Use timber and steel posts and offset blocks in accordance with Specification 967. Composite offset blocks may be substituted as approved on the APL. Use a single offset block type consistently per each run of guardrail. Steel offset blocks are only permitted for Modified Thrie Beam.

POST AND OFFSET BLOCK DETAILS

**REVISION** 11/01/17

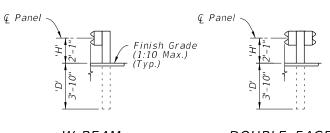
DESCRIPTION:

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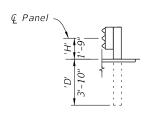
FY 2019-20 STANDARD PLANS

GUARDRAIL

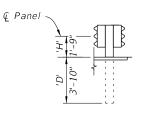
INDEX 536-001 SHEET



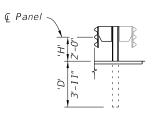
DOUBLE FACED W-BEAM W-BEAM



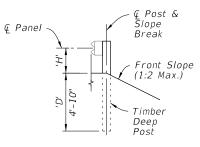
THRIE-BEAM



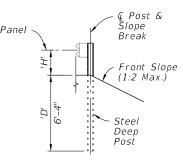
DOUBLE FACED THRIE-BEAM



*MODIFIED* THRIE-BEAM



SLOPE BREAK CONDITION TIMBER DEEP POST

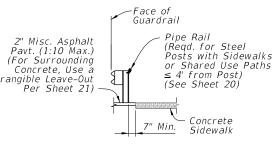


SLOPE BREAK CONDITION STEEL DEEP POST

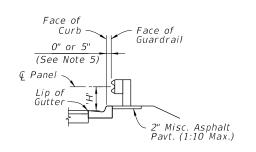
GUARDRAIL TYPES - MOUNTING HEIGHTS & POST DEPTHS:



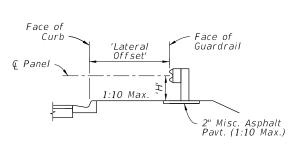
TYPICAL GRADING & PAVT. PLACEMENT DETAIL (See Note 2)



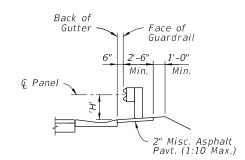
TYPICAL SIDEWALK DETAIL (Work with Other Sections as Read.)



ADJACENT TO CURB (Type F Curb Shown)



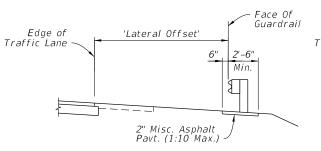
BEHIND CURB (Type F Curb Shown)



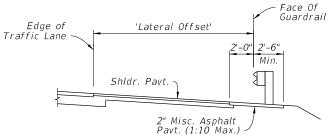
ADJACENT TO SHOULDER GUTTER

*GUARDRAIL SECTIONS - TYPICAL*=

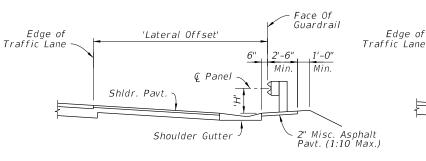
GUARDRAIL SECTIONS - CURB & GUTTER:



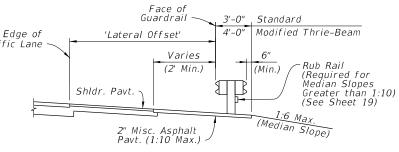
UNPAVED OR PARTIALLY PAVED SHOULDER



FULLY PAVED SHOULDER



SHOULDER GUTTER



DOUBLE FACED GUARDRAIL (Shown In Median)

#### GUARDRAIL SECTIONS - SHOULDERS:

GUARDRAIL HEIGHT SUMMARY TABLE:						
Туре:	Min. Depth 'D':	Mounting Height 'H':	Post Length 'L':			
W-Beam (Single and Double Faced)	3'-10"	2'-1"	6'-6"			
Thrie-Beam (Single and Double Faced)	3'-10"	1'-9"	6'-6"			
Modified Thrie-Beam	3'-11"	2'-0"	6'-9"			
Timber Deep Post	4'-10"	See Above	7'-6"			
Steel Deep Post	6'-4"	See Above	9'-0"			

#### NOTES:

- 1. GUARDRAIL SECTIONS: Construct Sections as indicated in the plans. The details shown herein depict W-Beam Guardrail, but are applicable to the other defined Guardrail Types placed at the corresponding height, 'H'. Use components per Sheets 4 & 5. Steel and timber post types are interchangeable unless otherwise defined. The 1:10 Max. cross slope shown is the maximum slope permitted for proper guardrail function, but project-specific cross slope requirements are governed per the plans.
- 2. TYPICAL GRADING & PAVEMENT PLACEMENT DETAIL: Construct features as depicted except where superceded by specific Guardrail Sections or the plans. Place the Slope Break a Minimum of 2' behind the post. For Deep Posts, the slope break may be placed at the @ Post with the 2" Miscellaneous Asphalt Pavement omitted.
- 3. SLOPE BREAK CONDITION: Install Deep Posts only where called for in the plans. Deep Posts are only permitted where post spacing is 6'-3" or less.
- 4. LATERAL OFFSETS: The Lateral Offsets shown are governed by the station and offset call outs for Face of Guardrail, as shown in the plans.
- 5. ADJACENT TO CURB: Place the Face of Guardrail consistently offset either flush with the Face of Curb or 5" behind the Face of Curb, as indicated by the plans station and offset callout. For offset changes, transition the Face of Guardrail as shown in the plans.

**GUARDRAIL SECTIONS** 

REVISION 11/01/17

DESCRIPTION:

FDOT

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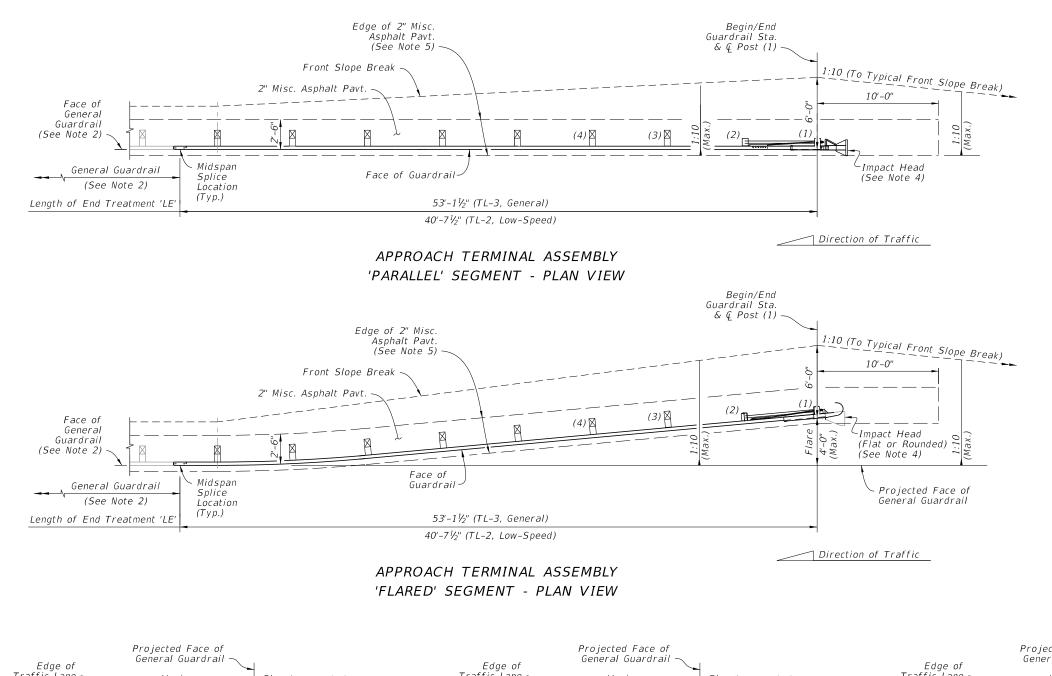
GUARDRAIL

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SHEET



NOTES:

1. INSTALLATION: Locate Approach Terminals where called for in the plans, with the Post (1) Q placed at the Begin/End Guardrail Station indicated in the plans.

The Plan Views shown herein are schematic only, showing basic geometry for Approach Terminals listed on the APL. The predefined Length of End Treatment, 'LE', includes the proprietary portion of various Approach Terminals and provides for more consistent planning of assembly installations across the differing Approach Terminal types. Forward-anchoring style Approach Terminals may vary from the planned lengths shown by up to 3'-0".

Construct Approach Terminals as shown in the APL and in accordance with the manufacturer's unique drawing details, procedures, and specifications.

Install posts in accordance with the manufacturer's drawings. The Special Posts on Sheet 21, including Special Steel Posts, Encased Posts, and Frangible Leave-Outs, are not permitted within the Approach Terminal segment unless otherwise called for in the plans.

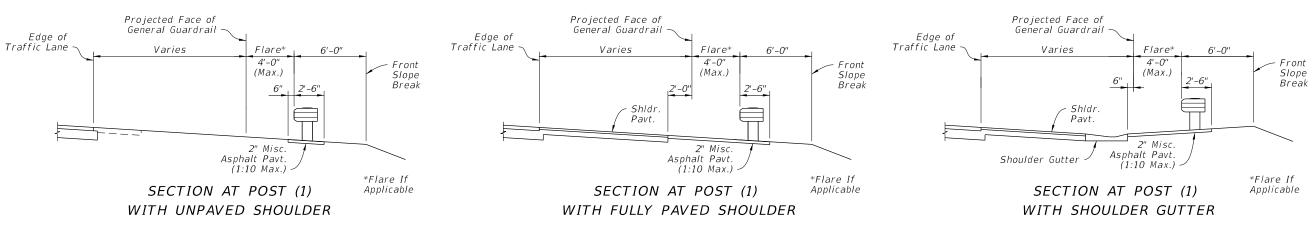
Align panel lap splices in accordance with the manufacturer's drawings, regardless of the direction of traffic.

Install adjacent grading, gutters, and/or curbing as shown herein, unless otherwise specified in the plans.

 GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments.

Approach Transitions, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.

- 3. APPROACH TERMINAL TEST LEVEL: Install either a Test Level 3 (TL-3) or Test Level 2 (TL-2) Approach Terminal as specified in the plans. TL-3 Approach Terminals may substitute for TL-2 Approach Terminals unless the substitution is specifically prohibited in the plans. TL-2 Approach Terminals may not substitute for TL-3 installations.
- 4. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification 536.
- 5. 2" MISCELLANEOUS ASPHALT PAVEMENT: The Plan Views shown herein depict the Unpaved Shoulder condition. For Fully Paved Shoulder and Shoulder Gutter conditions, extend the 2" Misc. Asphalt Pavement as shown in the corresponding 'Section at Post (1)' details below.
- 6. 'CURBED' AND 'DOUBLE FACED' GUARDRAIL SEGMENTS: See Sheet 8.



END TREATMENT -APPROACH TERMINAL GEOMETRY PARALLEL AND FLARED

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DESCRIPTION:

FDOT

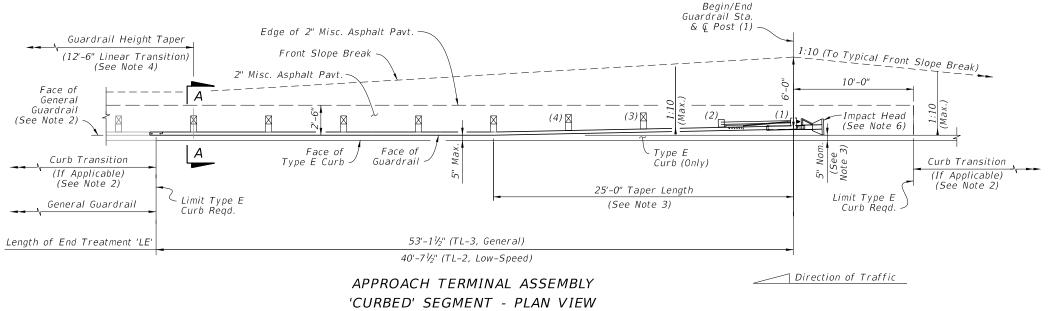
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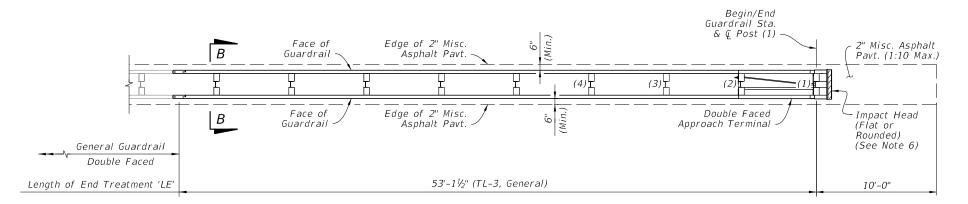
GUARDRAIL

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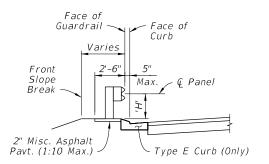
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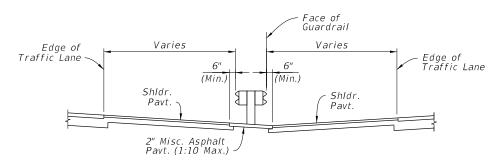




## APPROACH TERMINAL ASSEMBLY 'DOUBLE FACED' SEGMENT - PLAN VIEW



'CURBED' SECTION A-A (Height, 'H', Measured from Misc. Asphalt Pavt.)



'DOUBLE FACED' SECTION B-B (1:10 Slope or Flatter Regd.)

#### NOTES:

- 1. GENERAL: See Notes 1 through 3 on Sheet 7.
- 2. CURBED SEGMENTS: Type E curb is required within the limits shown. When a different curb type is called for outside of the Type E curb limits, transition the curb shape linearly, over a nominal distance ranging 5'-0" to
- 3. TAPER LENGTH: For Curbed Segments, taper the guardrail away from the roadway where shown to place the inside edge of the Impact Head at 5" behind the face of the curb. Where additional lateral offset is required to fit the Approach Terminal Assembly hardware, such as a soil plate, place the Impact Head as close to the curb as the hardware allows, not to exceed 2'-0" from the face of curb.
- 4. GUARDRAIL HEIGHT TAPER: For Curbed Segments, the connecting General Guardrail Mounting Height, 'H', is typically measured from the Lip of Gutter (See Sheet 6 Guardrail Sections, 'Adjacent to Curb'), while the End Terminal Assembly 'H' is measured from the Misc. Asphalt Pavt. (See Section A-A). Linearly taper the difference in Mounting Height over a minimum length of 12'-6", starting where indicated herein.
- 5. DOUBLE FACED SEGMENT: Connect to Double Faced General Guardrail. Use consistent Posts and Offset Block types as specified in the APL drawings over the entire Length of End Treatment, 'LE'. Posts and Offset Blocks in the adjoining General Guardrail segment may be different from those inside of the 'LE'. A change in post type between timber and steel is permitted, immediately outside of the 'LE' segment.

Maintain the 1:10 maximum grading as shown in Section B-B throughout segment 'LE'. Where required, transition to differing adjacent slopes linearly, over a minimum longitudinal length of 25'-0".

- 6. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification 536.
- 7. SINGLE FACED 'PARALLEL' AND 'FLARED' SEGMENTS: See Sheet 7.

END TREATMENT -APPROACH TERMINAL GEOMETRY CURBED AND DOUBLE FACED

**REVISION** 11/01/17

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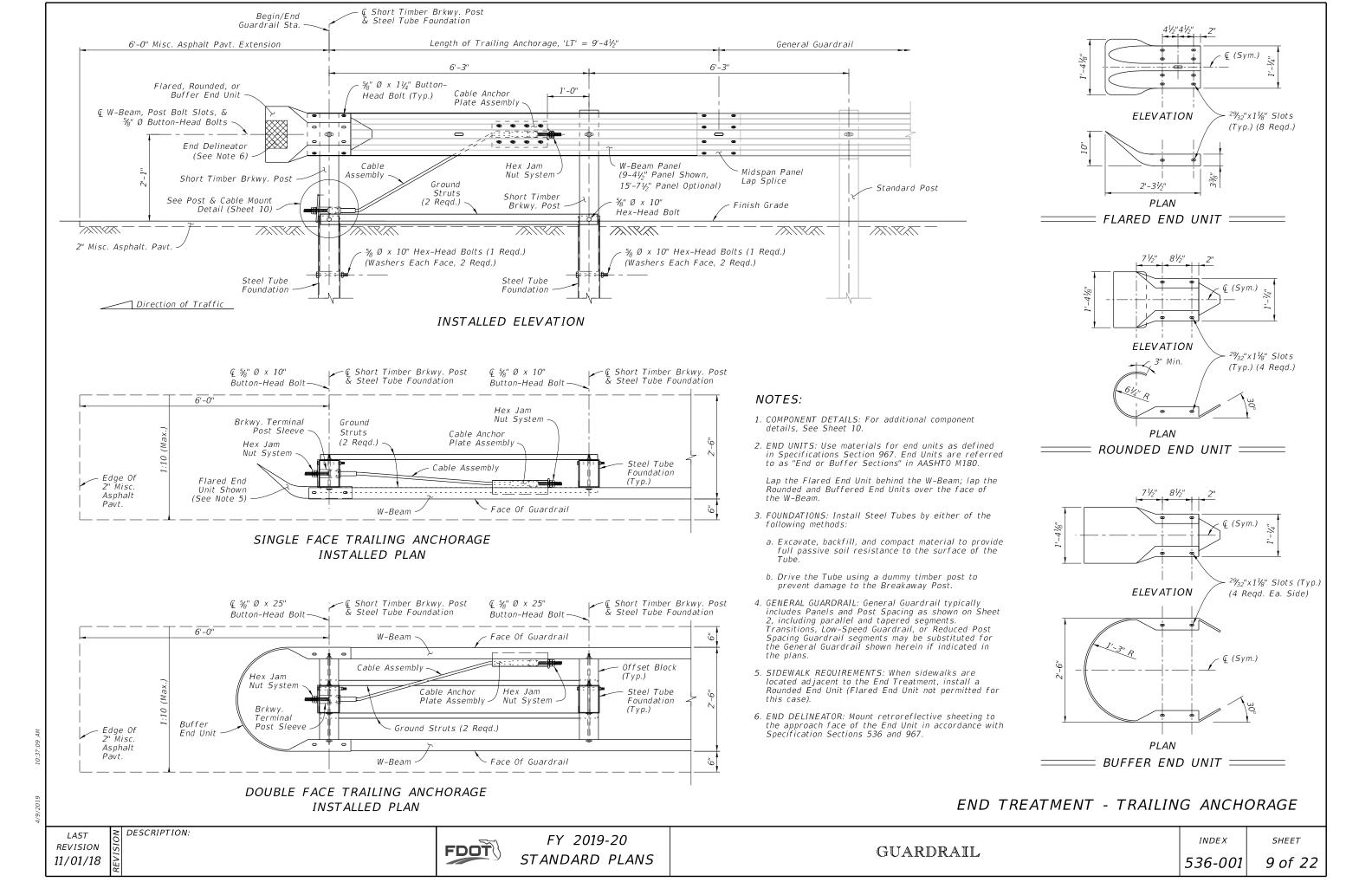
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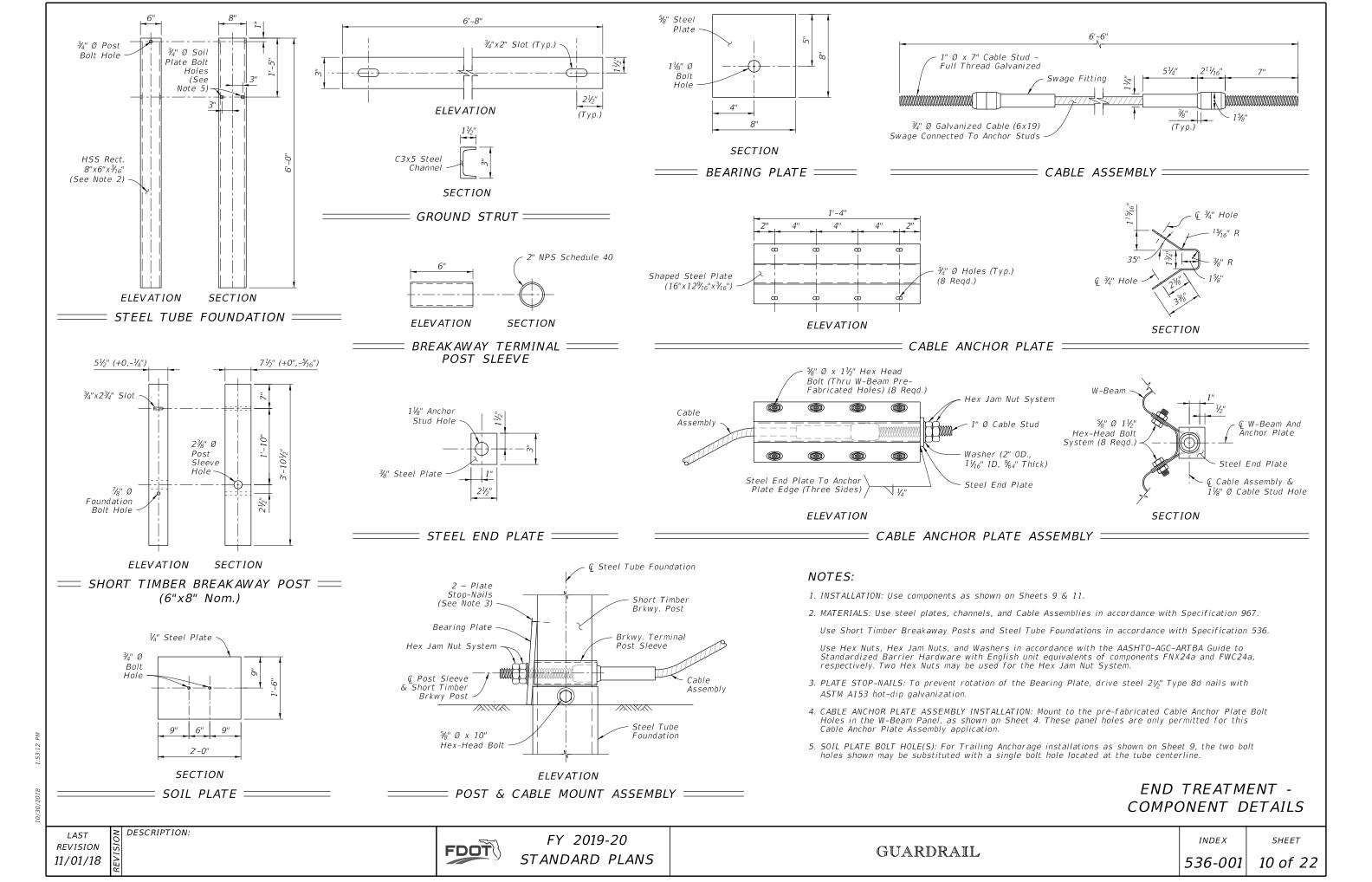
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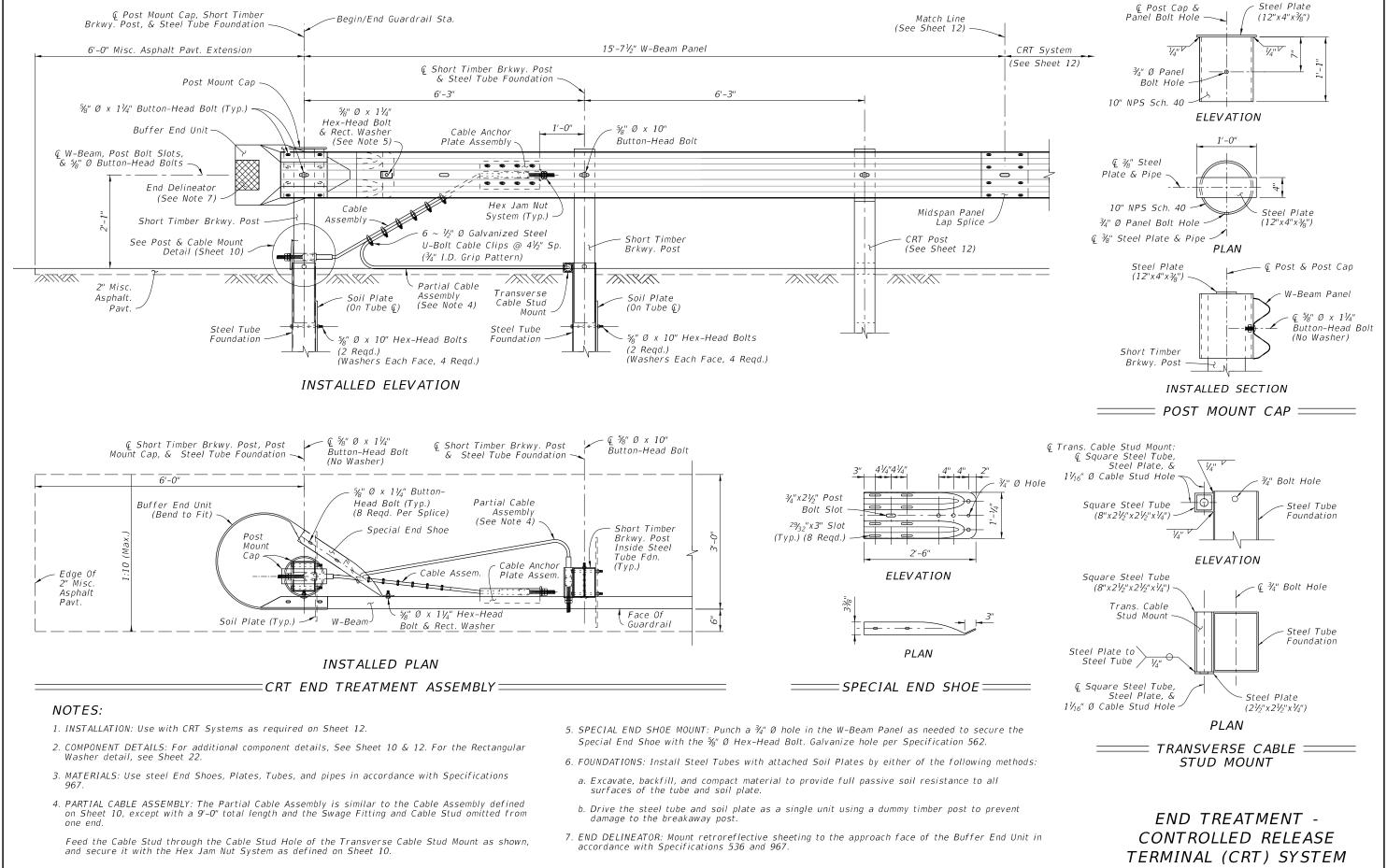
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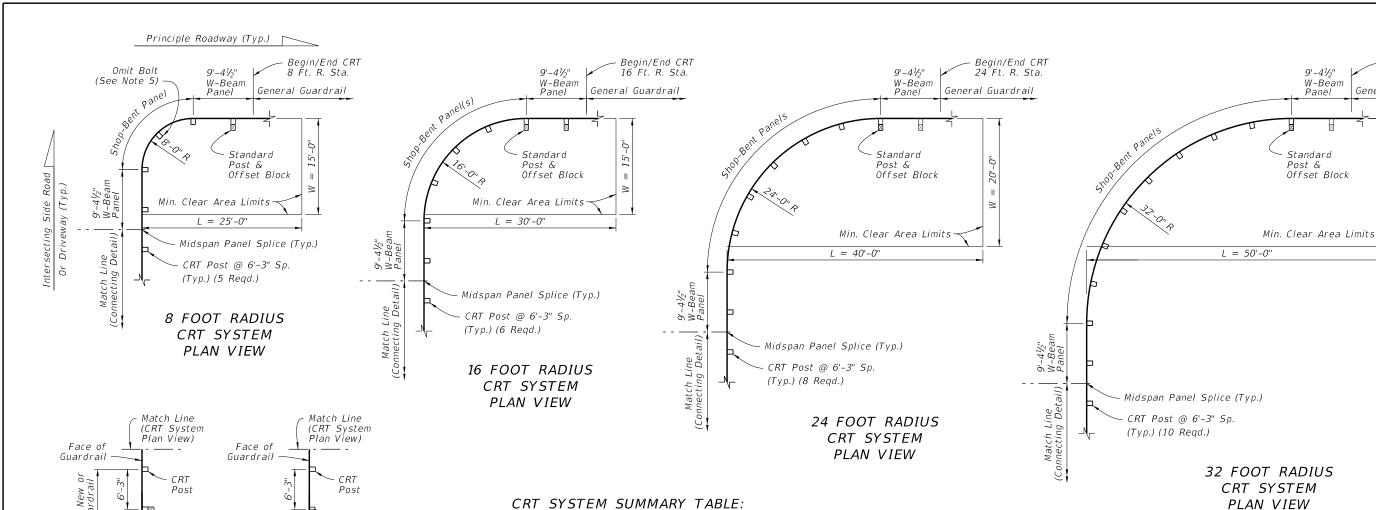
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RETURN RADIUS (FT.)	LENGTH OF SHOP-BENT PANEL(S) (FT.)	QUANTITY OF CRT POSTS	AREA CLEAR OF HAZARDS 'L' x 'W' (FT.)
8	12.5	5	25 x 15
16	25.0	6	30 x 15
24	37.5	8	40 x 20
32	50.0	10	50 x 20

CONTINUING OPTION

END TREATMENT OPTION

Begin/End

Guardrail Sta.

CONNECTING DETAIL

#### NOTES:

DESCRIPTION:

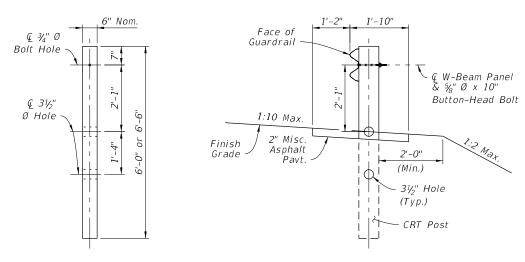
1. INSTALLATION: Construct the specified radius layout and Connecting Detail option as shown in the plans.

Treatment

Assembly

(See Sheet 11)

- 2. MIN. CLEAR AREA: Keep the area behind the CRT free of fixed objects and aboveground hazards within the Min. Clear Area limits shown. Maintain a slope not steeper than 1:10 for a minimum 2' behind the posts, and maintain a slope not steeper than 1:2 beyond 2'
- 3. APPROACH GRADING: Maintain grading on the roadway side of the guardrail face at a maximum slope of 1:10.
- 4. MATERIALS: For CRT Posts, use Timber Post material in accordance with Specification 967. Use steel panels and hardware in accordance with Specification 967.
- 5. BOLT OMISSION: For the 8 Foot Radius CRT System only, do not place a panel-to-post mount bolt at the center CRT Post (omit the \( \frac{\pi}{8} \)" Button-Head Bolt only at the location shown).
- 6. SHOP-BENT PANELS: Install Shop-Bent panel(s) where indicated using 12'-0" or 25'-0" W-Beam Panels. Splice at post locations within the CRT radius using the General configuration of  $\frac{9}{8}"$  Ø Button-Head Bolts (8 reqd. per splice).
- 7. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments. Approach Transitions, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.



CRT POST ELEVATION (6"x8" Nom. Timber)

CRT INSTALLED SECTION

LAYOUT FOR CONTROLLED RELEASE TERMINAL (CRT) SYSTEMS -SIDE ROADS AND DRIVEWAYS

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**FDOT** 

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Begin/End CRT 32 Ft. R. Sta.

General Guardrail

9'-41/2"

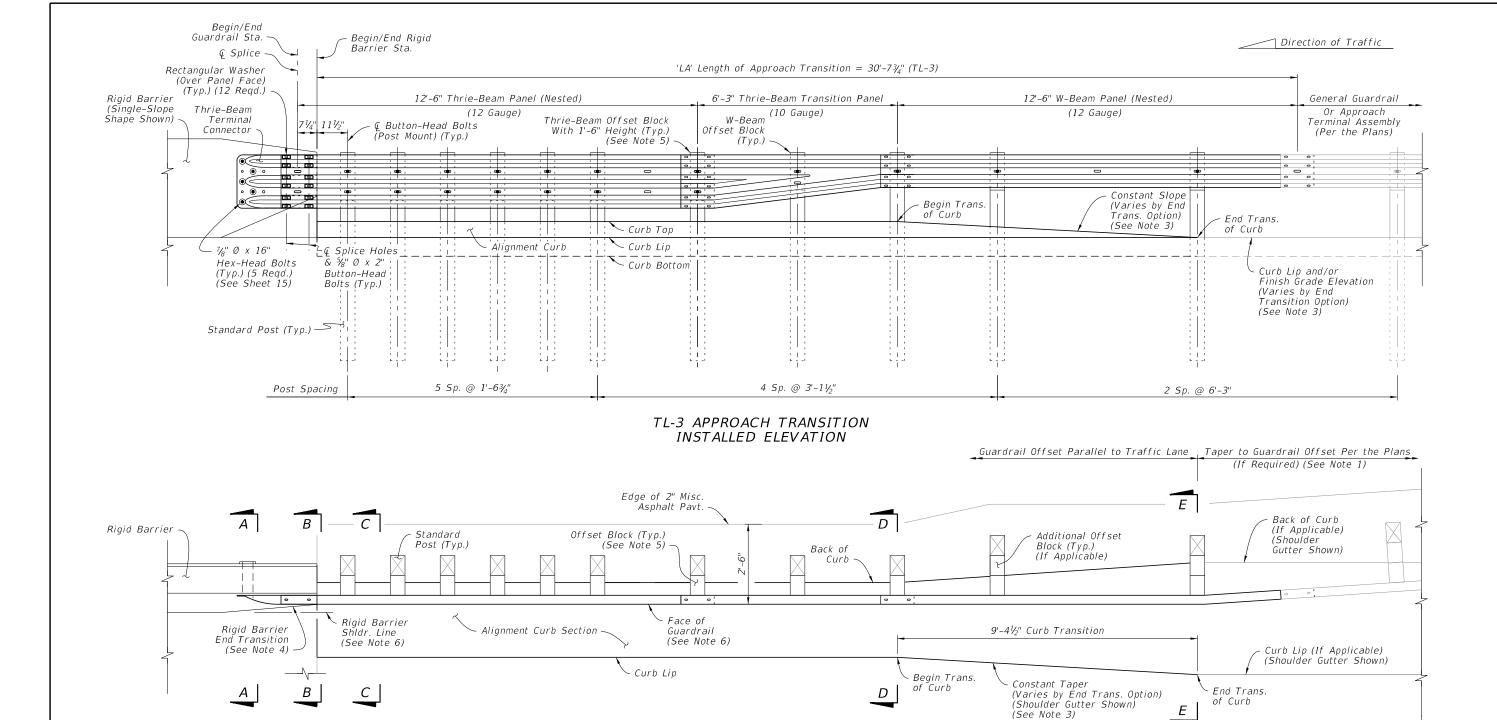
Panel

Standard

Offset Block

Post &

W-Beam



## NOTES:

1. INSTALLATION: Construct the Approach Transition segment where indicated in the plans. The required offset of the connecting adjacent guardrail is shown in the

The Layouts given on Sheet 17 provide basic schemes for connections to adjacent guardrail, where a taper to a differing guardrail offset may be required. If the adjacent guardrail segment has the same offset as the Approach Transition segment, then no taper is required.

For existing bridge connection options, see Indexes 536-002, 521-404, and 521-405.

2. SECTION VIEWS & DETAILS: For cross sections and details including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 15.

3. END TRANSITION OF CURB OPTION: The Plan and Elevation views depict an example Curb Transition to Shoulder Gutter from Section D-D to E-E, but this transition may require a different shape depending on the End Transition option indicated in the plans (Either a 'Shoulder Gutter Option', 'Raised Curb Option', or 'Flat No Curb Option'). See Sheet 15 for curb shape details.

TL-3 APPROACH TRANSITION

INSTALLED PLAN

- 4. RIGID BARRIER END TRANSITION: Taper the Rigid Barrier toe as shown. See Concrete Barrier, Index 521-001, and Traffic Railing, Indexes 521-422 and 521-428,
- 5. OFFSET BLOCKS: For Thrie-Beam post locations within the Length of Approach Transition segment, use the Timber Offset Blocks with 1'-6" height shown on Sheet 5.

For the midspan of the Thrie-Beam Transition Panel and for all other W-Beam locations shown herein, use the W-Beam Offset Blocks with 1'-2" height.

- 6. OFFSET: The required offset difference between the Face of Guardrail and Rigid Barrier Shoulder Line is considered negligible and may not be shown in the guardrail offset callouts in the plans. A consistent guardrail offset deviation of up to 4 inches outside of the Rigid Barrier Shoulder Line is permitted over the length 'LA'.
- 7. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments. Approach Terminals, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.

APPROACH TRANSITION CONNECTION TO RIGID BARRIER - GENERAL, TL-3

↑ Direction of Traffic

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DESCRIPTION:

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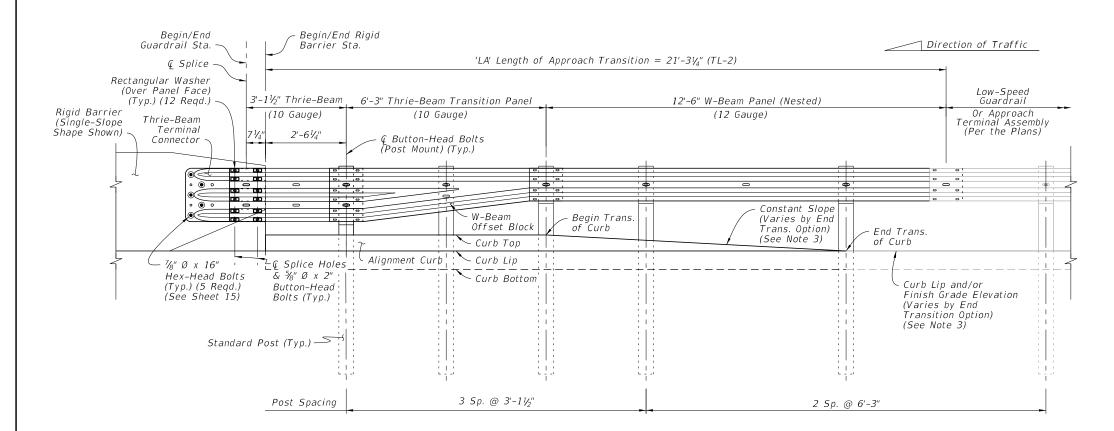
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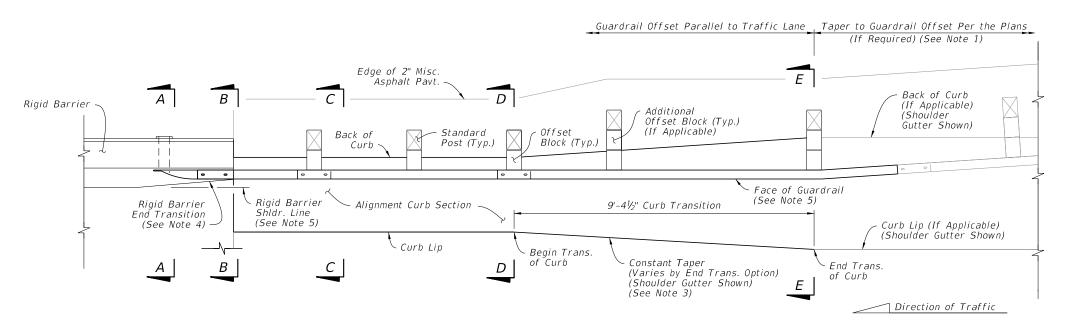
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#### TL-2 APPROACH TRANSITION INSTALLED ELEVATION



TL-2 APPROACH TRANSITION INSTALLED PLAN

#### NOTES:

1. INSTALLATION: Construct the Approach Transition segment where indicated in the plans. The required offset of the connecting adjacent guardrail is shown in the plans.

The Layouts given on Sheet 17 provide basic schemes for connections to adjacent guardrail, where a taper to a differing guardrail offset may be required. If the adjacent guardrail segment has the same offset as the Approach Transition segment, then no taper is required.

For existing bridge connection options, see Indexes 536-002, 521-404, and 521-405.

- 2. SECTION VIEWS & DETAILS: For cross sections and details including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 15.
- 3. END TRANSITION OF CURB OPTION: The Plan and Elevation views depict an example Curb Transition to Shoulder Gutter from Section D-D to E-E, but this transition may require a different shape depending on the End Transition option indicated in the plans (Either a 'Shoulder Gutter Option', 'Raised Curb Option', or 'Flat No Curb Option'). See Sheet 15 for curb shape details.
- 4. RIGID BARRIER END TRANSITION: Taper the Rigid Barrier toe as shown. See Concrete Barrier, Index 521-001, and Traffic Railing, Indexes 521-422 thru 521-428, for
- 5. OFFSET: The required offset difference between the Face of Guardrail and Rigid Barrier Shoulder Line is considered negligible and may not be shown in the guardrail offset callouts in the plans. A consistent guardrail offset deviation of up to 4 inches outside of the Rigid Barrier Shoulder Line is permitted over the
- 6. LOW-SPEED GUARDRAIL: Low-Speed Guardrail typically includes Panels and Post Spacing as shown on Sheet 3, including parallel and tapered segments. Approach Terminals, General Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the Low-Speed Guardrail shown herein if indicated in the plans.

APPROACH TRANSITION CONNECTION TO RIGID BARRIER - LOW-SPEED, TL-2

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DESCRIPTION:

**FDOT** 

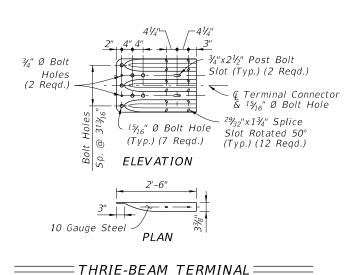
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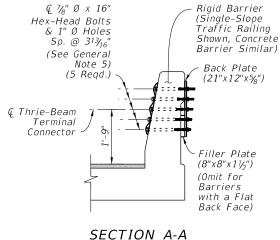
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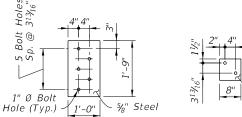
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RIGID BARRIER TERMINAL

CONNECTOR MOUNT



Alignment Curb

Begin Transition

Alignment Curb

Begin Transition (Section D-D)

Alignment Curb

Begin Transition (Section D-D)

(Section D-D)

SHOULDER GUTTER

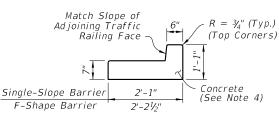
OPTION

RAISED CURB OPTION

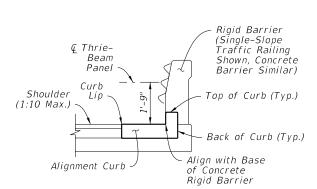
FLAT NO CURB OPTION

FILLER PLATE

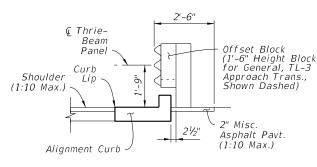




ALIGNMENT CURB SECTION



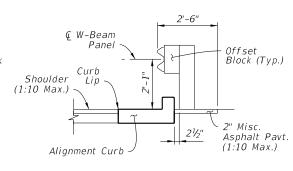
CONNECTOR DETAIL



SECTION C-C

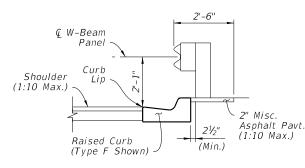
ALIGNMENT CURB

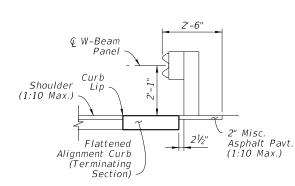
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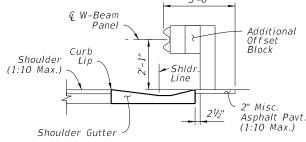
SECTION D-D BEGIN TRANSITION (End Alignment Curb)

SECTION B-B BEGIN ALIGNMENT CURB (Mate to Rigid Barrier)





SECTION E-E **END TRANSITION** FLAT NO CURB OPTION



SECTION E-E **END TRANSITION** RAISED CURB OPTION

**END TRANSITION** SHOULDER GUTTER OPTION

SECTION E-E

CURB TYPICAL SECTIONS

## NOTES:

- 1. PLAN AND ELEVATION VIEWS: Work with Sheets 13 & 14.
- 2. END TRANSITION OF CURB OPTION: Install one of the three End Transition types shown per Section E-E as indicated by the plans.

CURB TRANSITION ISOMETRIC VIEWS

- 3. GRADING BEHIND POSTS: Place Slope Break a Min. 2'-0" behind the post, per Sheet 6.
- 4. MATERIALS & CONSTRUCTION: Construct the concrete Aligning Curb and Curb transition in accordance with Specification 520. Use steel Plates and Thrie-Beam Terminal Connectors in accordance with Specification 967.

APPROACH TRANSITION CONNECTION - DETAILS

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**FDOT** 

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SHEET

Shoulder Gutter

Cross Section

Raised Curb

End Face

End Transition

(Section E-E)

End Transition

(Section E-E)

Cross Section

(Type-F Shown)

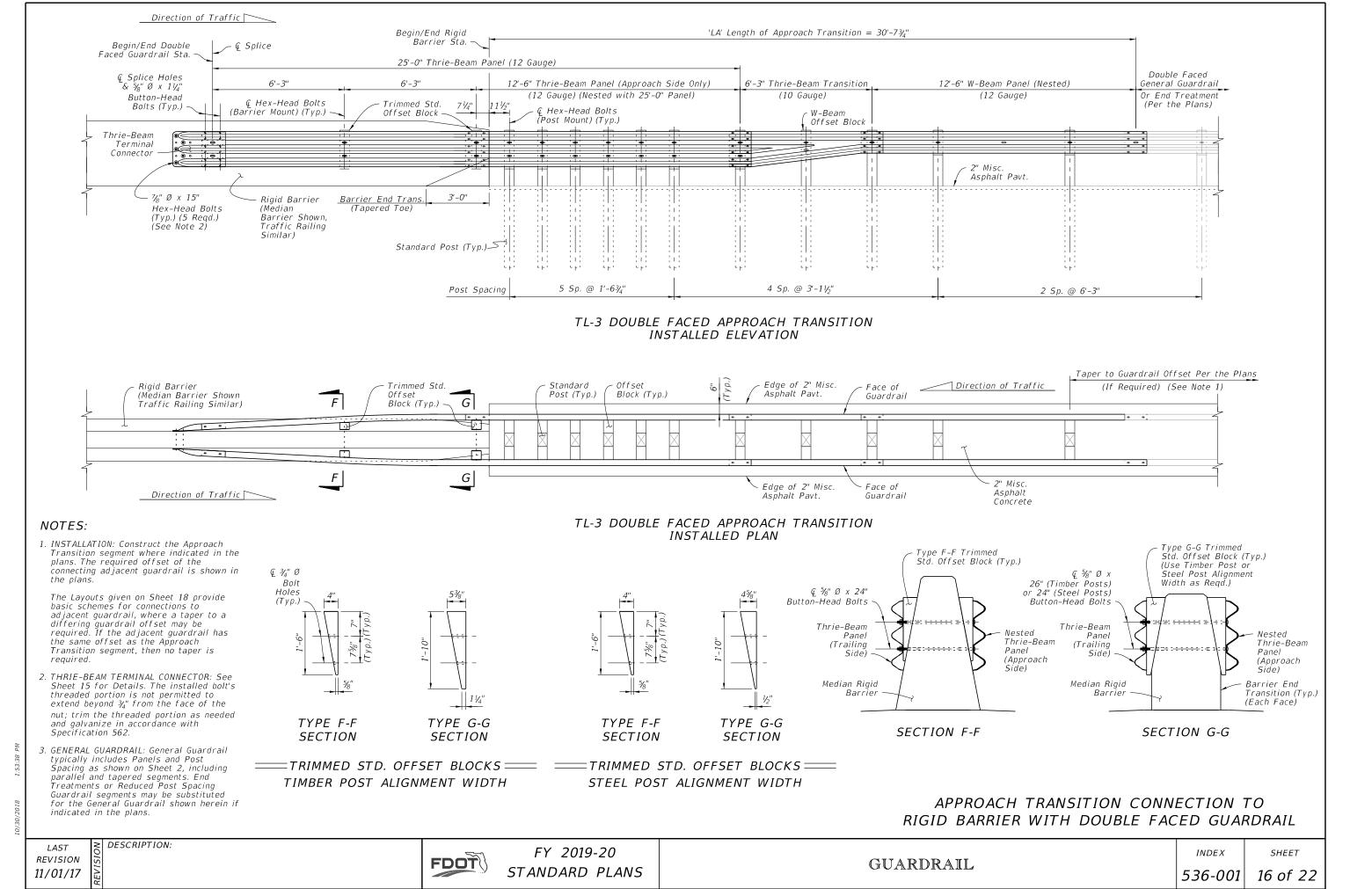
End Transition (Section E-E)

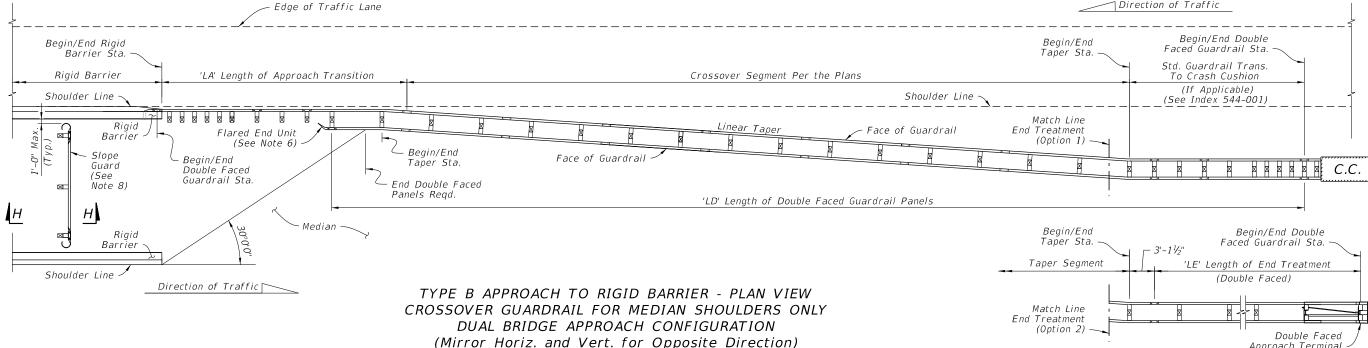
GUARDRAIL

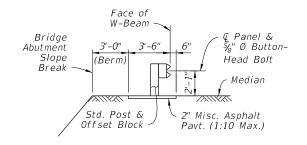
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DESCRIPTION:







SECTION H-H BRIDGE ABUTMENT SLOPE GUARD (Between Bridges)

DESCRIPTION:

#### NOTES:

- 1. INSTALLATION: The Plan Views shown are schematic only, showing example geometry for connecting quardrail segments including taper locations and Double Faced Guardrail requirements as applicable. Work this Sheet with the plans, where stationing and offsets for Begin/End Guardrail, Begin/End Rigid Barrier, and Begin/End Taper are specified. For existing bridge layouts, see Index 536-002, 521-404,
- 2. GENERAL (OR LOW-SPEED) GUARDRAIL SEGMENT: Construct this segment if shown in the plans. For the case where this segment's offset differs from the Approach Transition offset, linearly taper the guardrail between the Begin/End Taper Stations and offsets as specified in the plans

For the shortest length case of a direct connection between the End Treatment and the Approach Transition, this segment may be omitted as shown in the plans.

- 3. LENGTH OF APPROACH TRANSITION 'LA': Install the Approach Transition as shown per Sheet 13 or 14 as called for in the plans.
- 4. LENGTH OF END TREATMENT 'LE': Install the Approach Terminal End Treatment as shown per Sheet 7 or 8, where called for in the plans. Use the corresponding APL drawings for construction details.
- 5. CROSSOVER GUARDRAIL (FOR TYPE B APPROACH): Install the Crossover Segment tapering linearly from the Begin Taper Sta. and offset to the End Taper Sta. and offset as specified in the plans.

6. LENGTH OF DOUBLE FACED GUARDRAIL PANELS, 'LD' (FOR TYPE B APPROACH): Terminate the Double Faced Guardrail panels as shown (based upon the 30° line measured from the hazard on the opposite side of the median). Extend the panel segment longer than the dimension 'LD' as needed for the Panel's end Bolt Slot to align with a post Bolt hole.

Install a Flared End Unit where shown, as defined on Sheet 9.

- 7. END TREATMENT OPTIONS (FOR TYPE B & C APPROACH): For Double Faced applications, use either a Double Faced Approach Terminal Assembly per Sheet 8 or a Crash Cushion per Index 544-001. For either Option, meet the 1:10 adjacent grading requirements for Approach Terminals as shown on Sheet
- 8. SLOPE GUARD: Where indicated in the plans, install a Guardrail segment between bridge approaches and offset from the bridge abutment's Slope Break as shown. Install posts at the end bolt slots of the panel system. Use post spacing of either 3'-11/3" or 6'-3", as needed to correctly fit system between barriers. The system may also be lengthened to fit by installing two Rounded End Units as defined on Sheet 9.

## LAYOUT TO RIGID BARRIER -APPROACH ENDS

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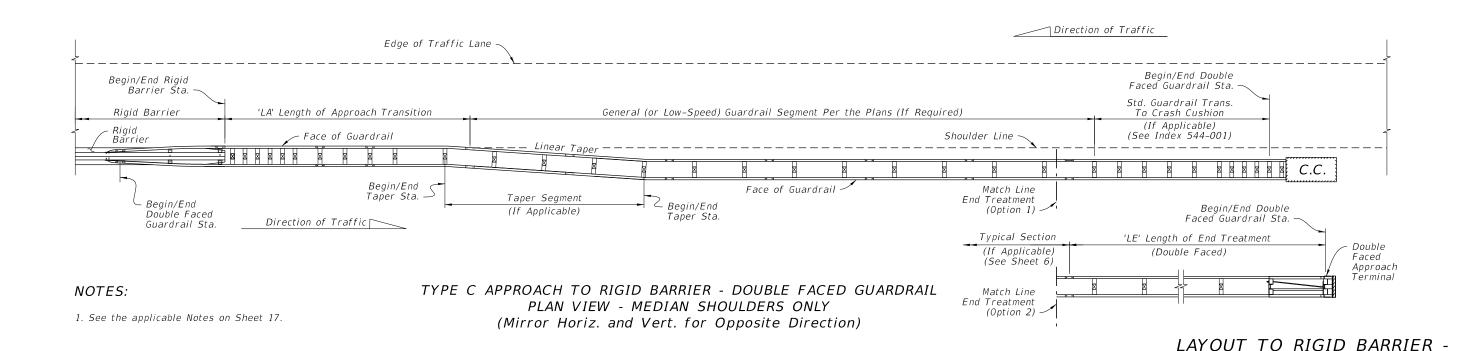
GUARDRAIL

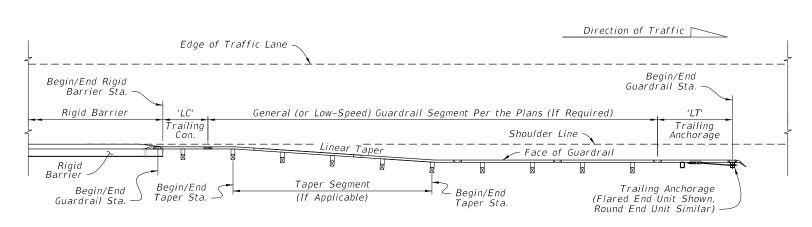
INDEX

Approach Terminal

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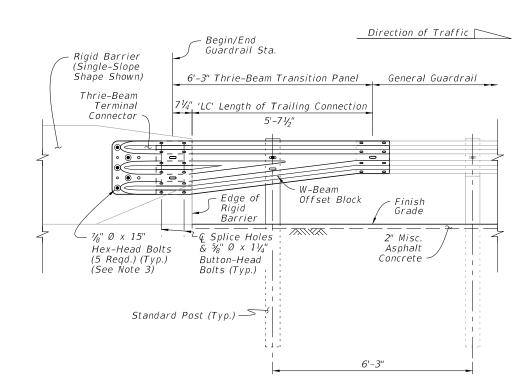


TYPE D TRAILING CONNECTION FROM RIGID BARRIER PLAN VIEW - MEDIAN OR OUTSIDE SHOULDER (Mirror Horiz. and/or Vert. for Opposite Direction and/or Side of Road)

#### NOTES:

DESCRIPTION:

- 1. See the applicable Notes on Sheet 17.
- 2. LENGTH OF TRAILING ANCHORAGE, 'LT': Install the Trailing Anchorage as shown on Sheet 9, where called for in the plans.
- 3. THRIE-BEAM TERMINAL CONNECTOR: Install connector and bolts as shown on Sheet 15.
- 4. RIGID BARRIER SINGLE SLOPE END FACE: See Concrete Barrier Wall, Index 521-001, and Traffic Railing, Indexes 521-422 and 521-423, for details.



TRAILING END TRANSITION CONNECTION TO RIGID BARRIER - INSTALLED ELEVATION

> LAYOUT TO RIGID BARRIER -TRAILING ENDS

APPROACH ENDS WITH DOUBLE FACED GUARDRAIL

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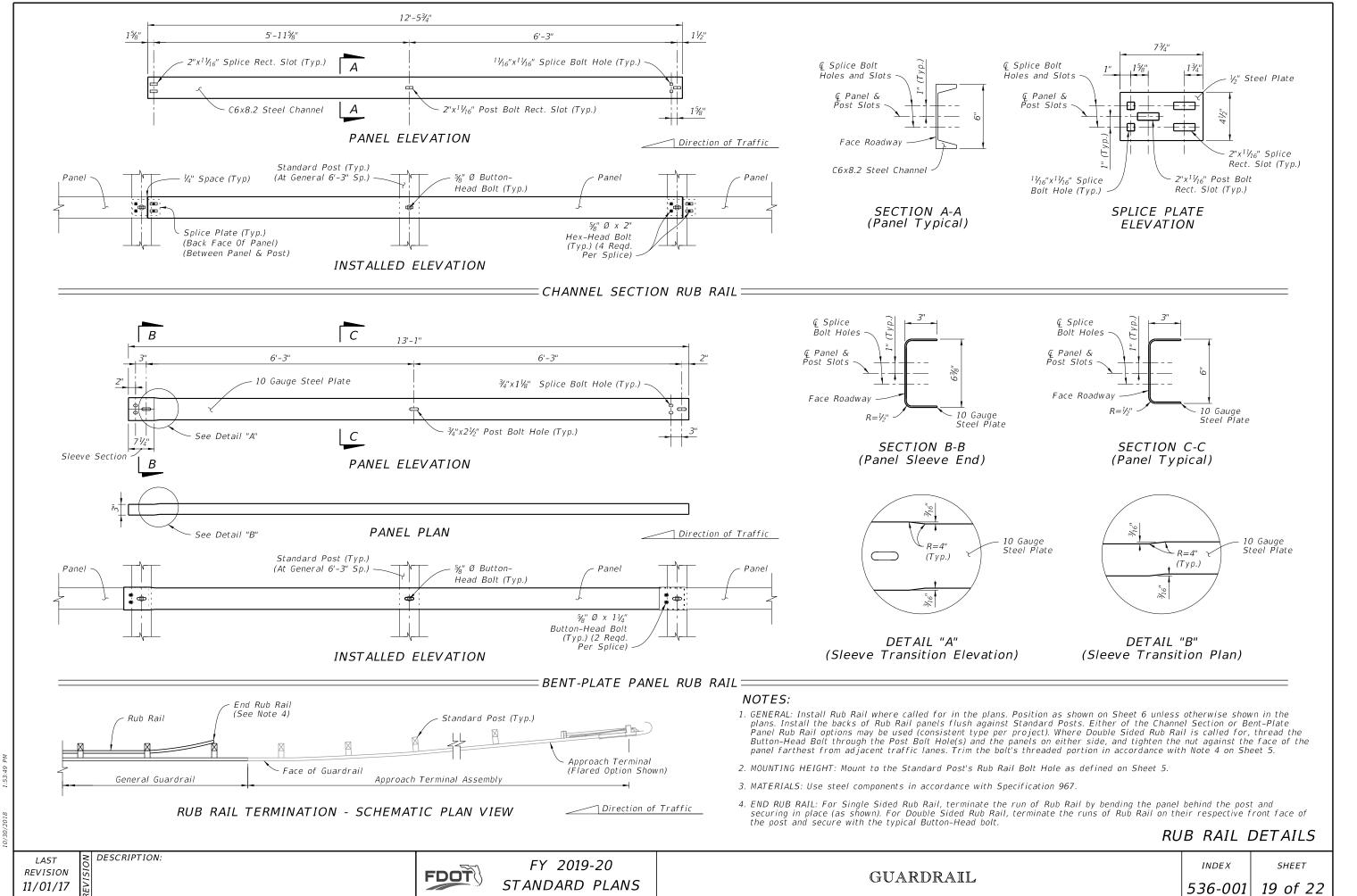
**FDOT** 

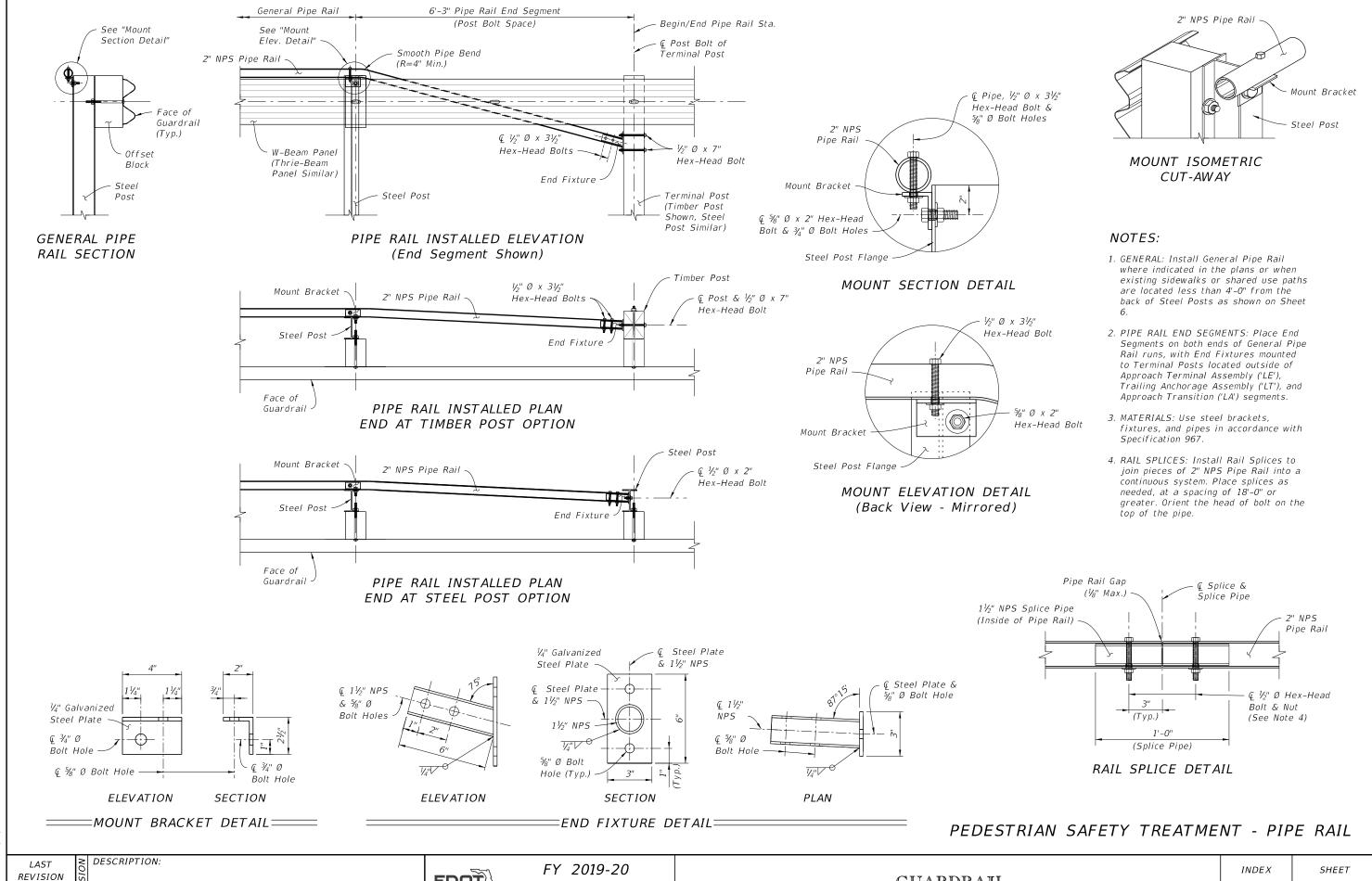
FY 2019-20 STANDARD PLANS

GUARDRAIL

*INDEX* 536-001

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11/01/17

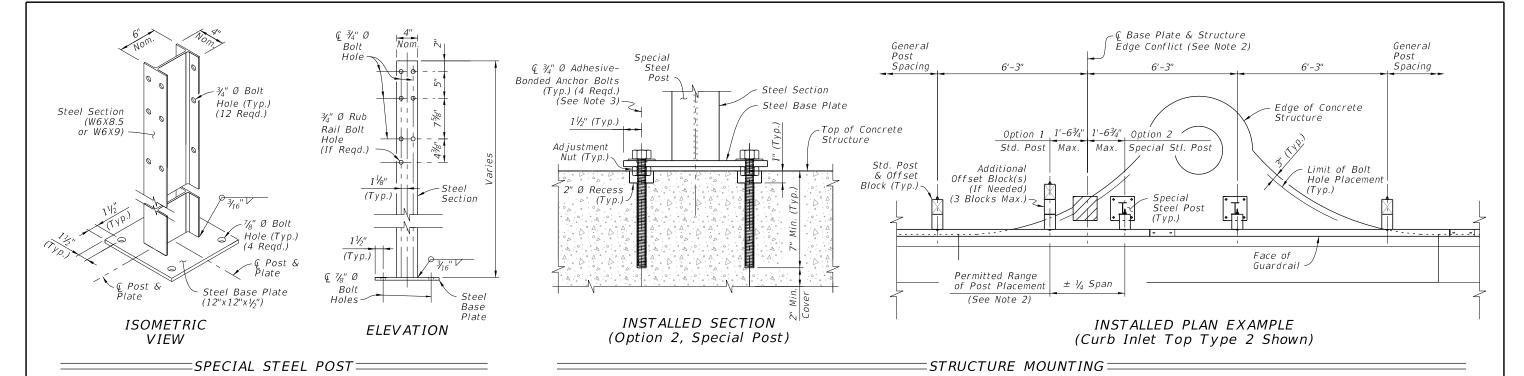
FDOT

STANDARD PLANS

GUARDRAIL

536-001

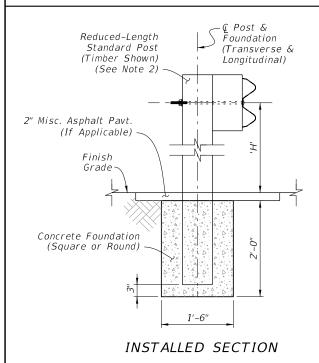
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#### **NOTES:**

- 1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) located atop culverts, inlets, pier footings, or similar concrete structures, a Special Steel Post may be substituted for a Standard Post. Install where shown in the plans and/or as-needed, in accordance with Specification 536.
- 2. EDGE CONFLICT: When a required post location causes an Edge Conflict with the structure, where the Steel Base Plate is not located entirely on the structure at least 3" from the Edge of Concrete, the longitudinal post location may be altered by up to 1'-6¾" (Quarter Span) from the original required spacing location to prevent the Edge Conflict. With the post location adjusted, use a Std. Post mounted in soil (Option 1) or a Special Steel Post with its Base Plate mounted entirely on the structure (Option 2). Maintain the original required spacing locations upstream and downstream
- 3. BASE PLATE MOUNT: Install Special Steel Posts as shown using steel Adhesive-Bonded Anchor Bolts in accordance with Specifications 536. Use 3/4" Hex-Head Bolts for structures less than 9" deep as defined in the Specification.
- 4. PANEL MOUNT TO ADJUSTED POST: Punch additional ¾"x2½" Post Bolt Slot(s) in the W-Beam or Thrie-Beam Panel only where needed to mount the panel to a post in an adjusted location. Meet the Panel Post Bolt Slots requirements of Specification 536.
- 5. MATERIALS: Use steel base plates in accordance with Specification 536.

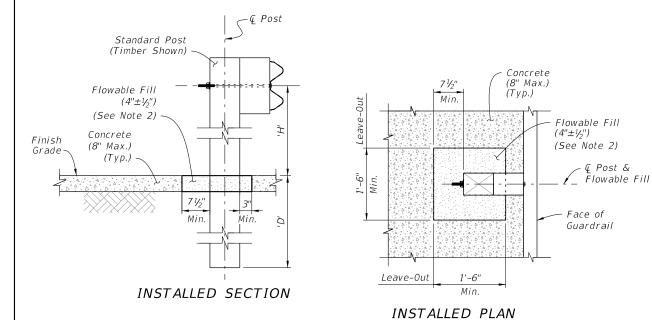
## SPECIAL STEEL POST FOR CONCRETE STRUCTURE MOUNT



DESCRIPTION:

#### NOTES:

- 1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) conflicting with underground utilities or other underground obstructions, an Encased Post may be used where a 2'-0" depth will avoid the conflict. Install where shown in the plans and/or as-needed, in accordance with Specification 536.
- 2. REDUCED-LENGTH STANDARD POST: Use a Standard Post with reduced Length such that the Panel Height 'H' is maintained while the post bottom terminates 3" from the bottom of the Concrete Foundation. Typically, the Post Length 'L' is 4'-7" for W-Beam Guardrail.
- 3. FOUNDATION: Use non-reinforced Class NS Concrete material in accordance with Specification 347. After casting the concrete, ensure the surrounding soil material is completely backfilled and tamped to provide full passive resistance.
- 4. LIMIT: Encased Posts are not permitted for consecutive posts unless otherwise shown in the plans.



#### NOTES:

1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) placed within a concrete surface (typically a sidewalk), use a Frangible Leave-Out around the post base as shown. Install where shown in the plans and/or as-needed, in accordance with Specification 536.

For the required 1'-6" x 1'-6" Leave-Out, smoothly cut the existing concrete surface or form-up the square shape when an application has new surrounding concrete.

Ensure Flowable Fill surface is smooth and even with the adjacent concrete

2. MATERIALS: Use Non-Excavatable Flowable Fill in accordance with Specification 121, not to exceed 150 psi.

## FRANGIBLE LEAVE-OUT FOR CONCRETE SURFACE MOUNT

**REVISION** 11/01/17

ENCASED POST FOR SHALLOW MOUNT

**FDOT** 

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*536-001* 

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- 2. MATERIALS: Use materials of the size and type defined for Barrier Delineators in Specifications 993.
- 3. COLOR: Use either white or yellow retroreflective sheeting to match the color of the nearest lane's edgeline.
- 4. MOUNT LOCATIONS: Mount Barrier Delineators atop posts as shown, starting with Post (3) of Approach Terminals and incrementally increasing spacing towards the downstream direction. Install the Barrier Delineators at the following

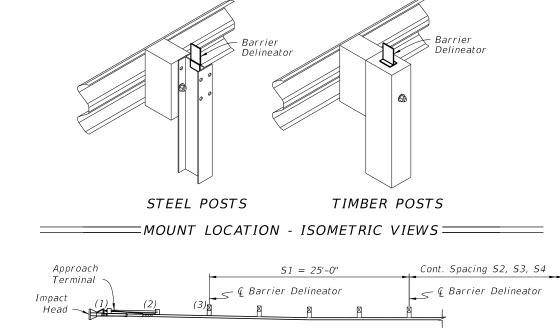
 $S1 = 25' \times 1 \ Space$ 

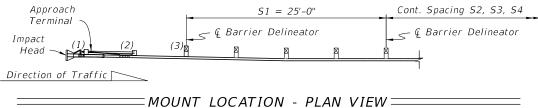
 $S2 = 50' \times 1 \ Space$  $53 = 75' \times 1 \text{ Space}$ 

 $S4 = 100' \times for$  the Remaining Run

Additionally, place a Barrier Delineator on Post (2) of the Trailing Anchorage or on the post nearest the Rigid Barrier.

5. MEDIAN GUARDRAIL: Install retroreflective sheeting on both sides of the barrier delineator for Guardrail on medians.





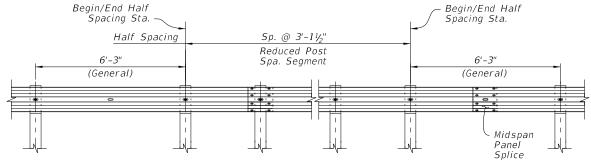
### BARRIER DELINEATORS

#### NOTES:

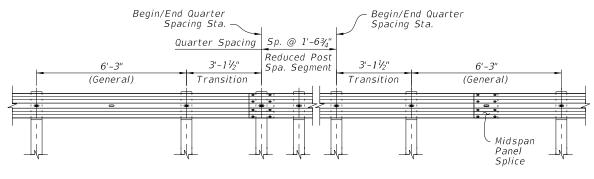
1. INSTALLATION: Work these details with the plans, where Stationing for Begin/End Half Spacing and Begin/End Quarter Spacing are indicated if required.

Where the Begin/End Stations indicated in the plans do not correspond exactly to post locations in construction, extend the Reduced Post Spacing segment to the nearest post(s) before the Begin Station and/or after the End Station called for

- 2. PANEL SPLICES: Midspan Panel Splices are not required in Transition and Reduced Post Spacing segments, however they are required for General segments. To place midspan splices in General segments, use one Non-General panel length (9'-41/2" or 15'-71/2") or add an additional Transition spaced post where required.
- 3. LOW-SPEED GUARDRAIL: For Reduced Post Spacing with Low-Speed Guardrail (12'-6" post spacing), the Reduced Spacing pattern requires a 6'-3" space between the 12'-6" and 3'-11/3"
- 4. PANEL POST BOLT SLOTS: For Quarter Spacing configurations, punch additional 3/4"x21/2" Post Bolt Slots in the panels only where required for mounting and in accordance with Specification 536.

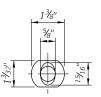


DETAIL 'S' - HALF SPACING ELEVATION (AS REQD. PER THE PLANS)

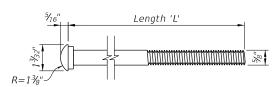


DETAIL 'S' - QUARTER SPACING ELEVATION (AS REQD. PER THE PLANS)

## REDUCED POST SPACING FOR HAZARDS





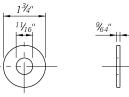


**ELEVATION** OPTION 1

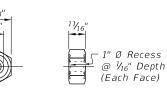
**ELEVATION** OPTION 2

**PROFILE** (Option 1 Shown)

BUTTON-HEAD BOLT =



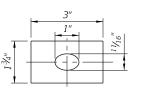




ELEVATION PROFILE ==WASHER ====

ELEVATION **PROFILE** 

===HEX-NUT =====





**ELEVATION** 

**PROFILE** 

 $\equiv$  RECTANGULAR WASHER  $\equiv$ (For CRT & Terminal Connectors Where Shown -Install Over Panel Face)

### BUTTON-HEAD BOLT LENGTHS:

Application(s):	Length 'L':	Min. Thread Length:
Panel Splice	1 1/4"	Full Length
Steel Post Mount - Single Faced Guardrail	10"	4"
Timber Post Mount - Single Faced Guardrail	18"	4"
Steel or Timber Post Mount - Double Faced Guardrail	25"	4"
Modified Thrie-Beam Panel / Terminal Connector Splice	2"	Full Length

#### NOTES:

- 1. Use nuts, bolts, and washers in accordance with Specification 967.
- 2. For Steel Posts with Double Faced Guardrail, the single 25" Length bolt (one bolt thru both post flanges) may be replaced with two 10" Length bolts (one bolt per post flange).
- 3. Use bolts listed in Table 2 in corresponding locations shown in this Index.

5/8" BUTTON-HEAD BOLT SYSTEM

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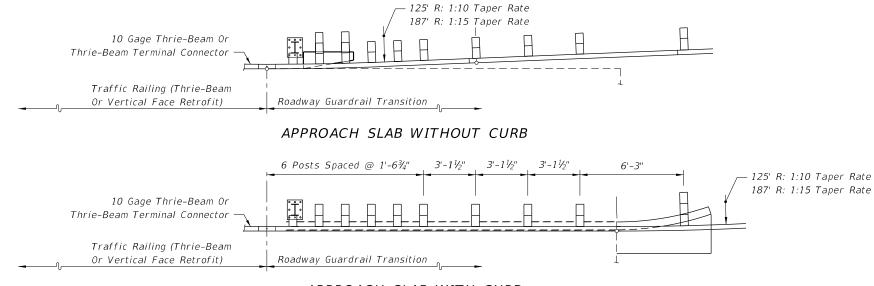
GUARDRAIL

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# SPECIAL STEEL POST FOR ROADWAY THRIE-BEAM TRANSITIONS TO BRIDGE TRAFFIC RAILING RETROFITS



#### APPROACH SLAB WITH CURB

Longitudinal Location Of Transition Blocks And Curb End Flares Will Vary With Scheme Type

#### PARTIAL PLAN VIEWS

#### GENERAL NOTES

- 1. This index provides guardrail transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 26 apply to bridges with retrofitted traffic railings, (Sheet 26 shows the trailing end guardrail connections). Sheet 27 applies to bridges with safety shaped traffic railing. Construct the guardrail transitions and connections where shown in the plans.
- 2. For trailing end guardrail connections for existing bridges with either Vertical Face Retrofits or Safety Shape Traffic Railing, see the Trailing End Transition Connection to Rigid Barrier detail shown in Index 536-001. Likewise, for miscellaneous guardrail construction details that are not provided in this Index, refer to Index 536-001.

## NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- 1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.
- 2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to curb blunt ends are not in place.
- 3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated in accordance with Specification 967.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification 937 and be installed in accordance with Specification 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

- 5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 15 through 18 and the flag notation on Sheet 26.
- 6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT

LAST REVISION 11/01/17 Au

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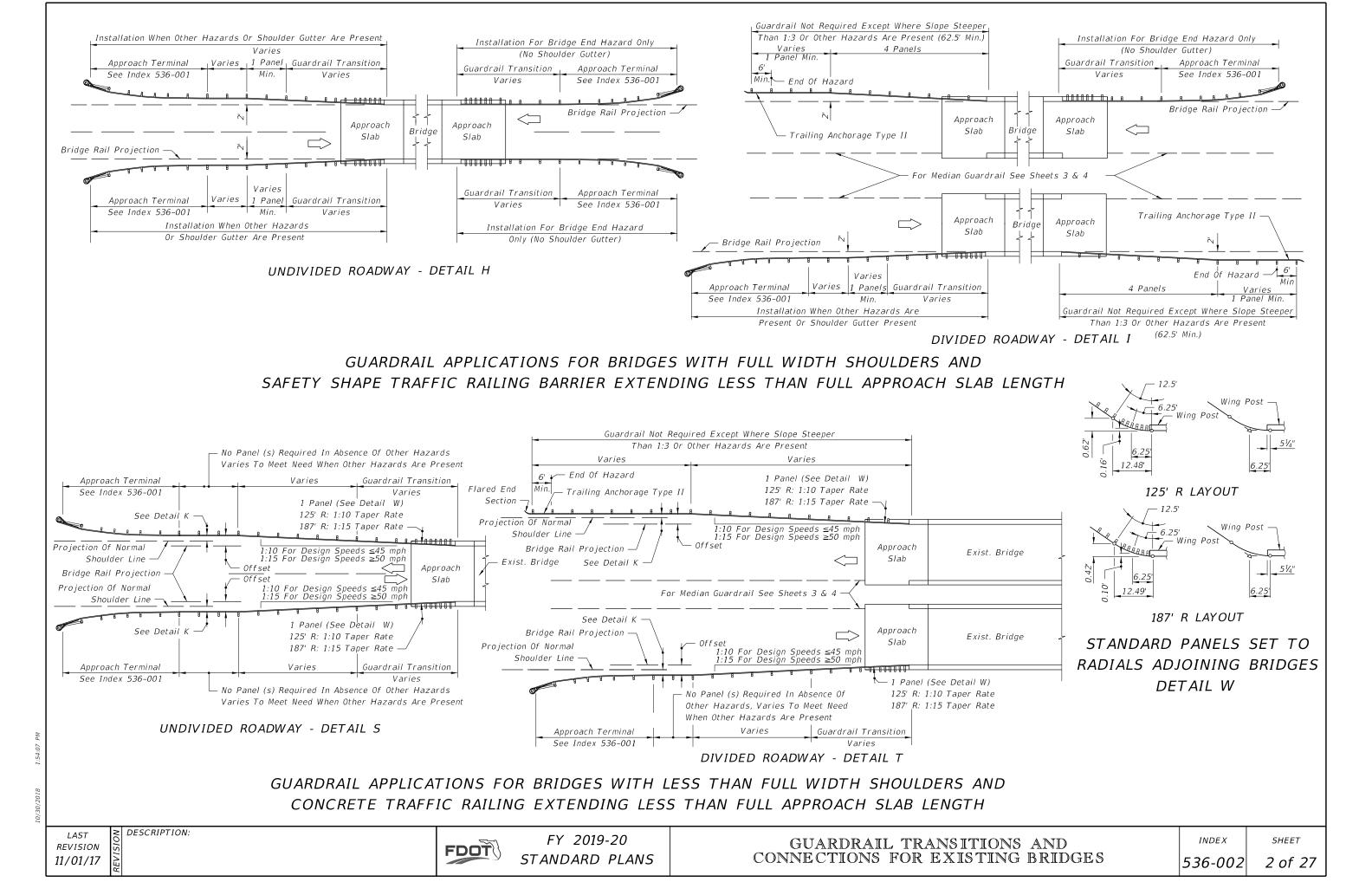
FY 2019-20 STANDARD PLANS

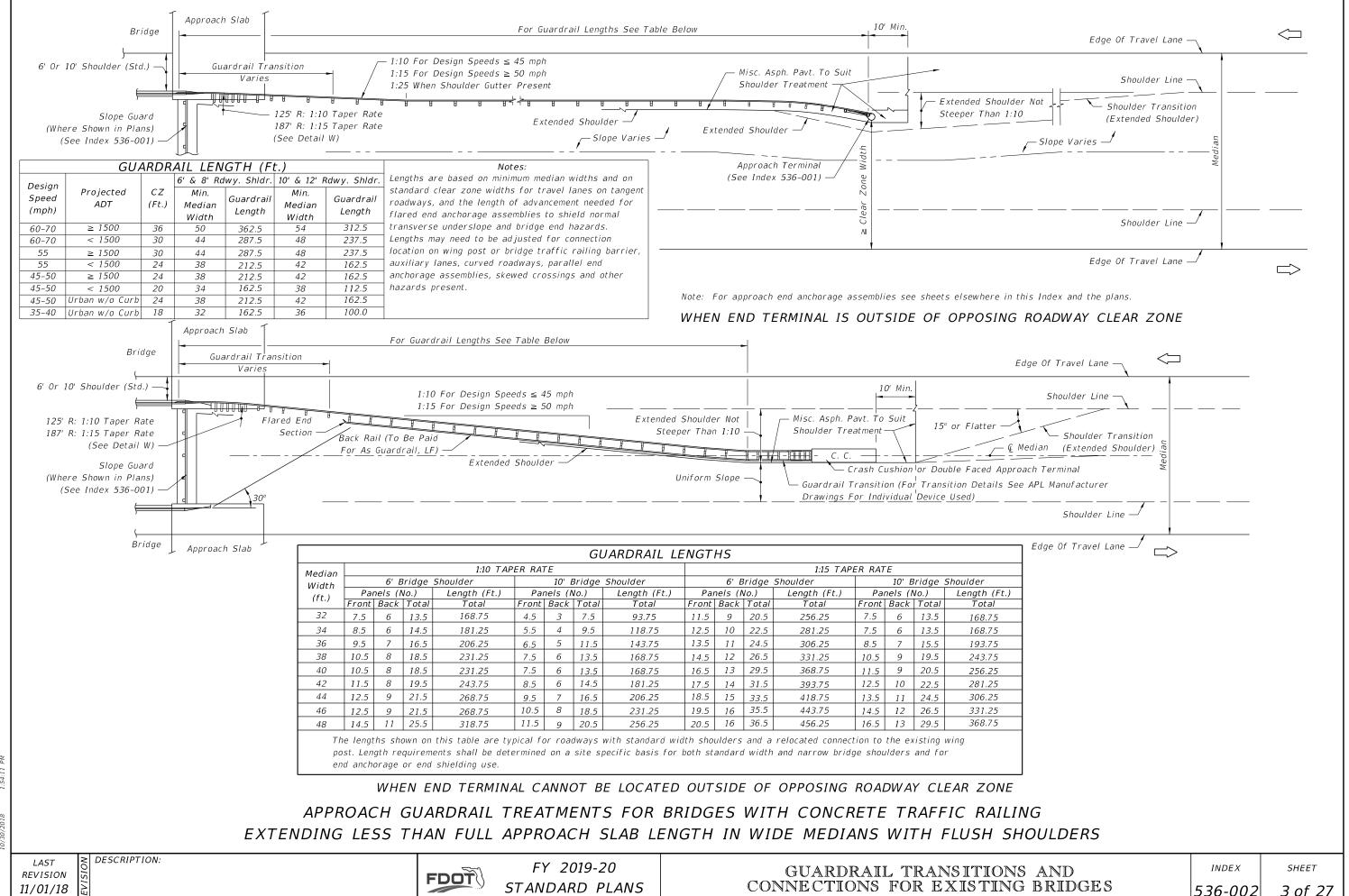
GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

INDEX

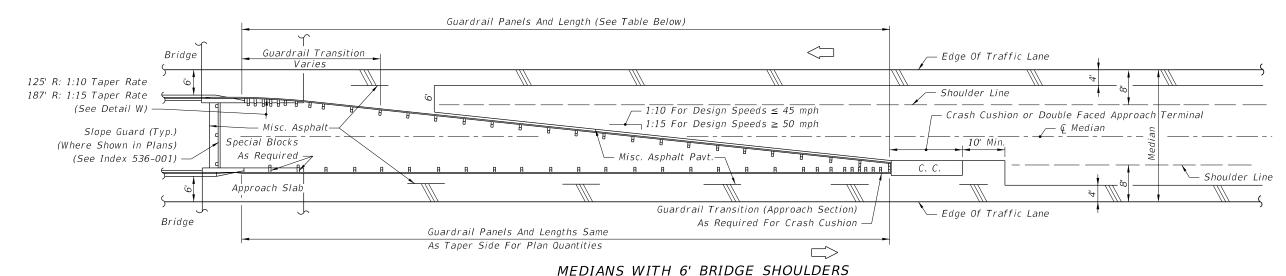
SHEET

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#### MEDIANS WITH 10' BRIDGE SHOULDERS



Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

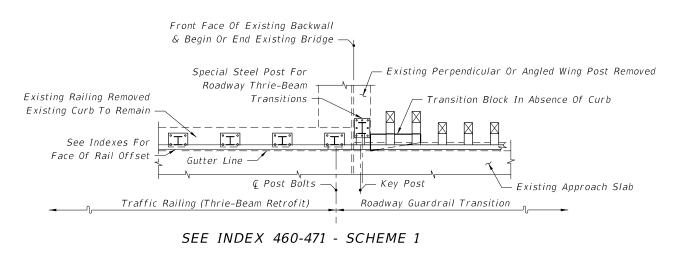
GUARDRAIL LENGTHS								
MEDIAN	6' BRIDGE SHOULDERS			10' BRIDGE SHOULDERS				
WIDTH			1:15 TAPER RATE		1:10 TAPER RATE		1:15 TAPER RATE	
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	12.5	156.25	18.5	231.25	6.5	81.25	9.5	118.75
28	11.5	143.75	16.5	206.25	5.5	68.75	7.5	93.75
26	9.5	118.75	14.5	181.25	5.5*	68.75	5.5*	68.75
24	8.5	106.25	11.5	143.75	5.5*	68.75	5.5*	68.75

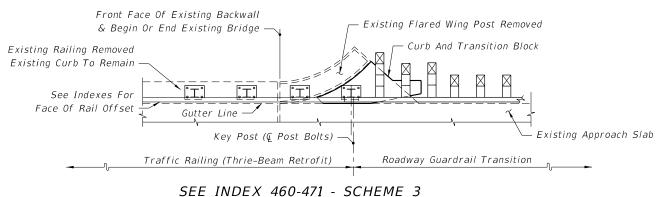
The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see * below.

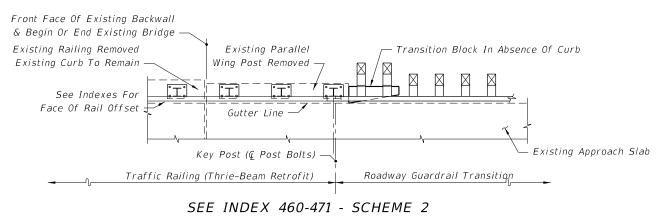
*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

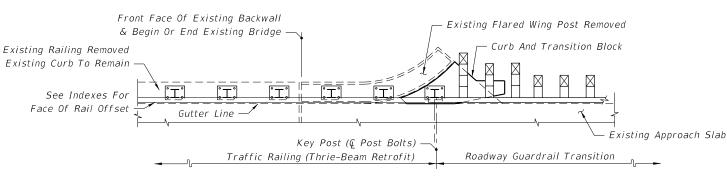
## APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH CONCRETE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

DESCRIPTION:









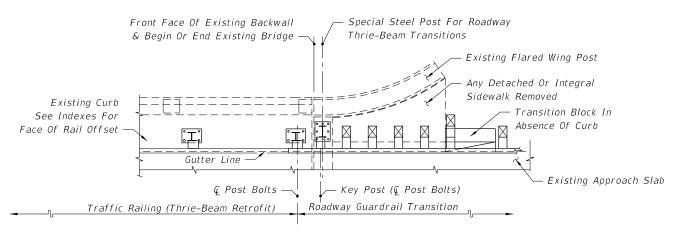
SEE INDEX 460-471 - SCHEME 3

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

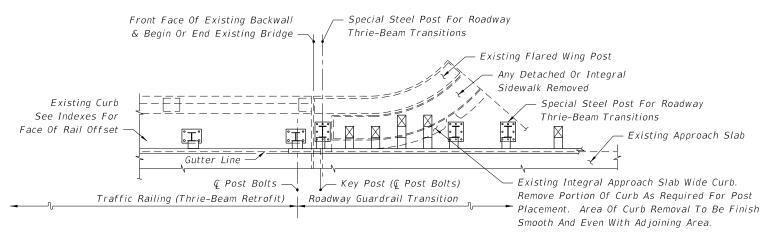
**REVISION** 11/01/17

DESCRIPTION:

## SEE INDEXES 460-472 & 460-475 - SCHEME 2



SEE INDEXES 460-472 & 460-475 - SCHEME 2



SEE INDEXES 460-472 & 460-475 - SCHEME 2

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS
FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

10/30/2018 1:54:21

LAST REVISION 11/01/17

DESCRIPTION:

Front Face Of Existing Backwall

Gutter Line

Traffic Railing (Thrie-Beam Retrofit)

Existing Curb

See Indexes For

Face Of Rail Offset

& Begin Or End Existing Bridge —

© Post Bolts -

SEE INDEXES 460-472 & 460-475 - SCHEME 1



- Existing Perpendicular Or Angled Wing Post

Transition Block In Absence Of Curb

- Existing Approach Slab

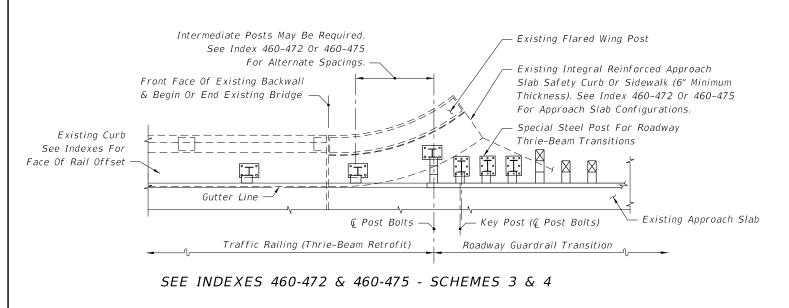
Special Steel Post For Roadway

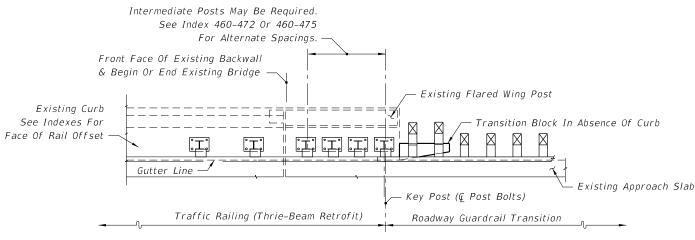
Thrie-Beam Transitions

– Key Post (& Post Bolts)

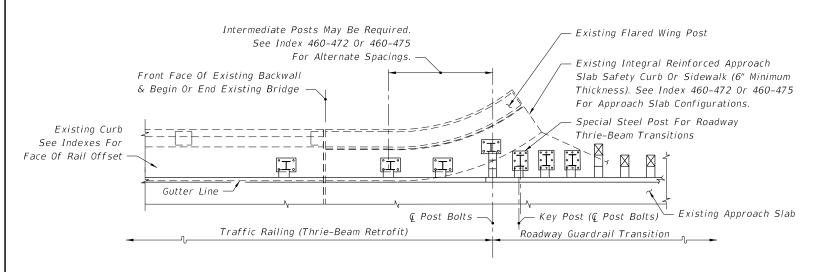
Roadway Guardrail Transition

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SEE INDEXES 460-472 & 460-475 - SCHEMES 5 & 6



Intermediate Posts May Be Required. See Index 460-472 Or 460-475 For Alternate Spacings Existing Flared Wing Post Front Face Of Existing Backwall & Begin Or End Existing Bridge End of Existing Approach Slab Flare Existing Curb See Indexes For Transition Block In Absence Of Curb Face Of Rail Offset Gutter Line ─ - Existing Approach Slab Key Post (© Post Bolts) Traffic Railing (Thrie-Beam Retrofit) Roadway Guardrail Transition

SEE INDEXES 460-472 & 460-475 - SCHEMES 3 & 4

SEE INDEXES 460-472 & 460-475 - SCHEMES 5 & 6

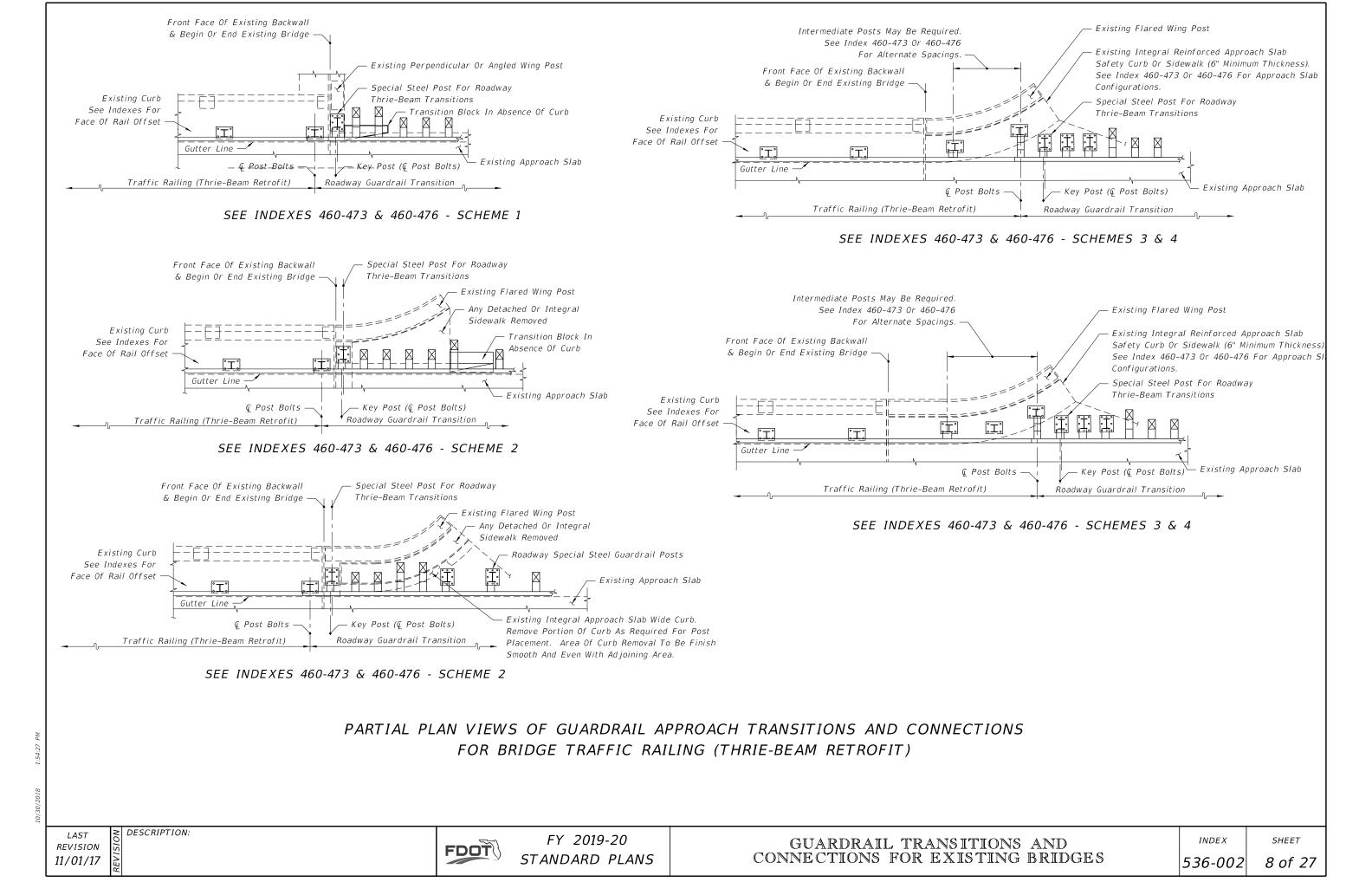
PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS
FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

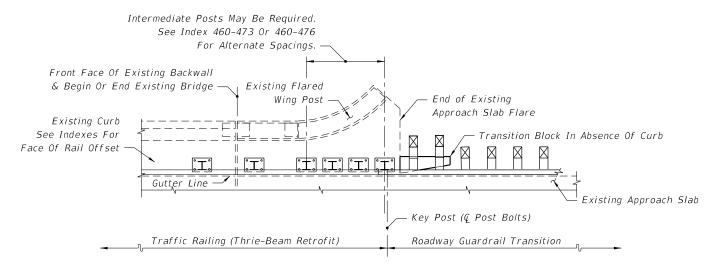
10/30/2018 1:5

LAST REVISION 11/01/17

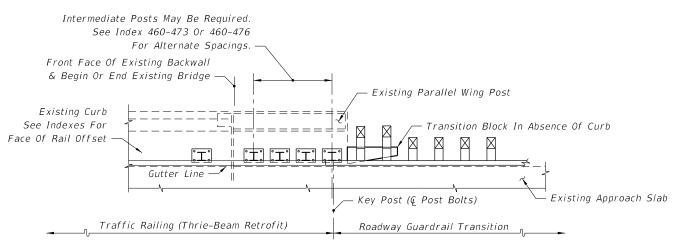
DESCRIPTION:

FDOT





SEE INDEXES 460-473 & 460-476 - SCHEMES 5 & 6



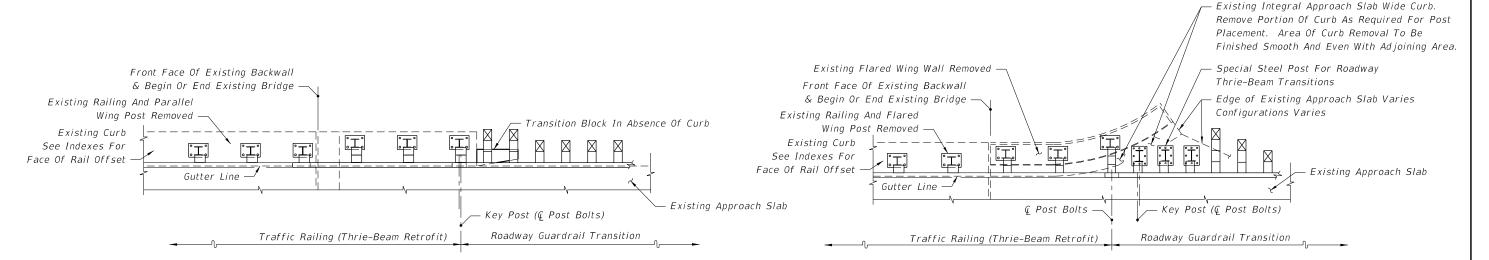
SEE INDEXES 460-473 & 460-476 - SCHEMES 5 & 6

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

DESCRIPTION:

SEE INDEX 460-474 - SCHEME 1

SEE INDEX 460-474 - SCHEME 2



SEE INDEX 460-474 - SCHEME 3

SEE INDEX 460-474 - SCHEME 3

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

**REVISION** 11/01/17

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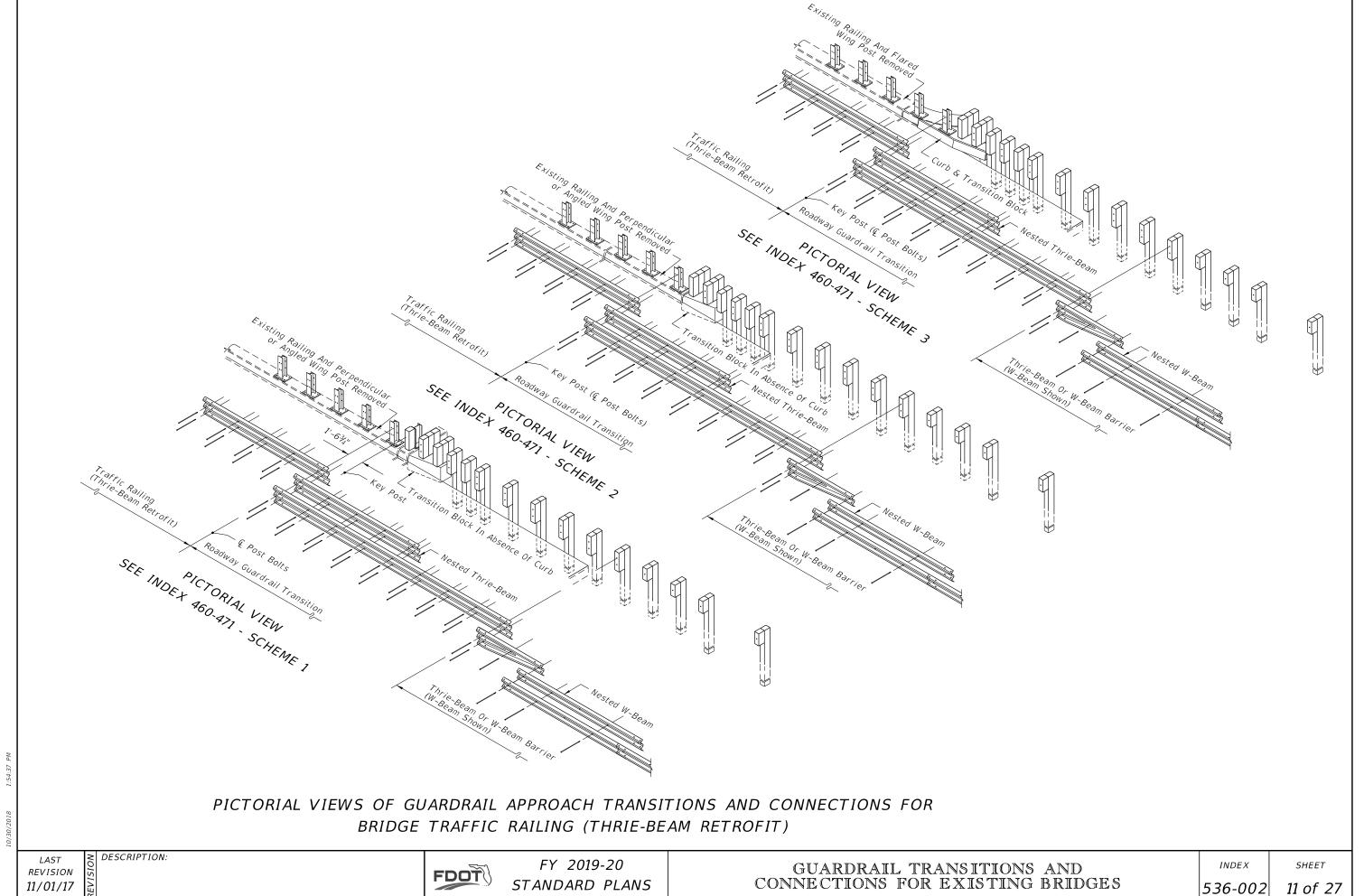
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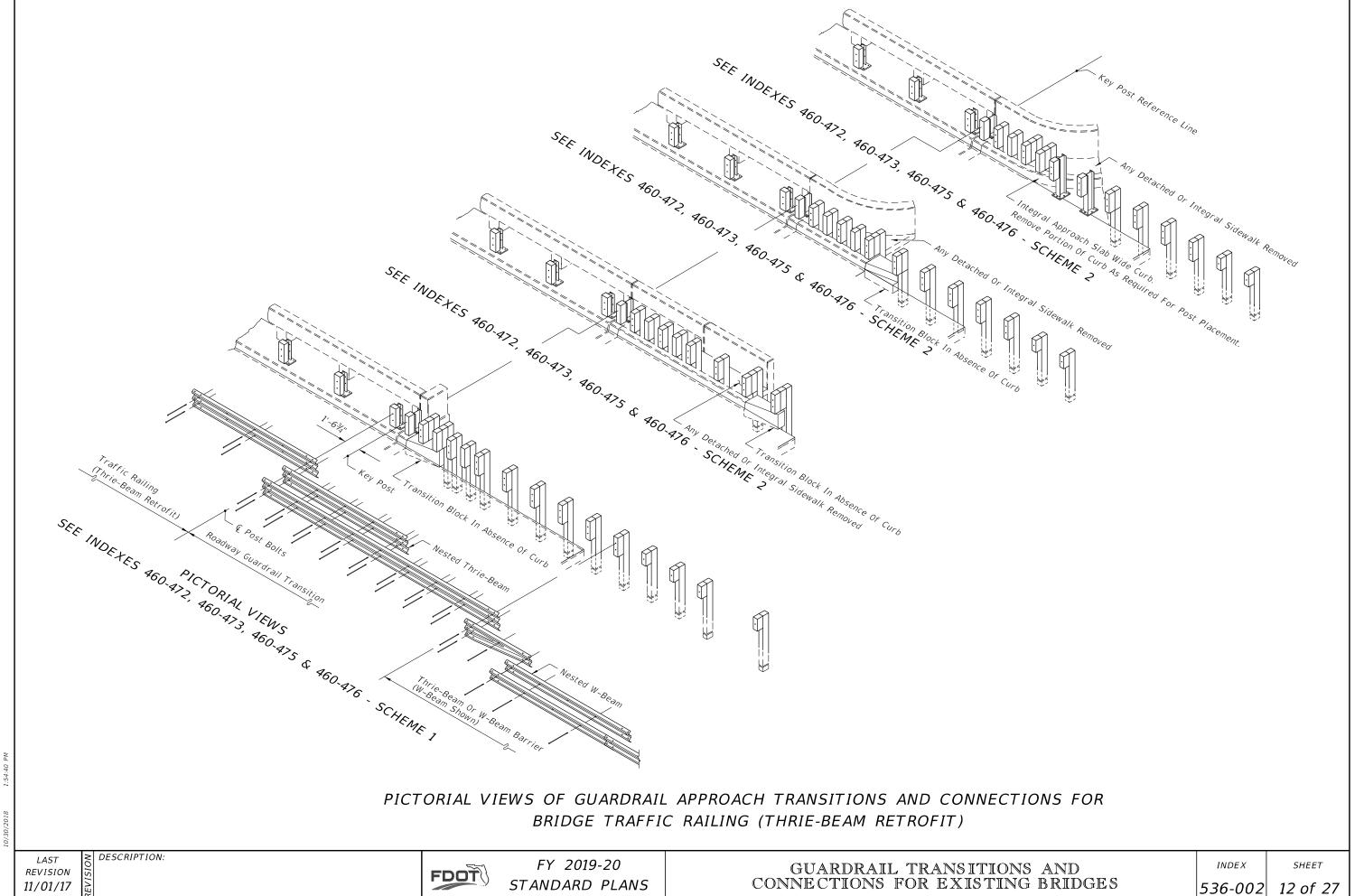
FY 2019-20 STANDARD PLANS

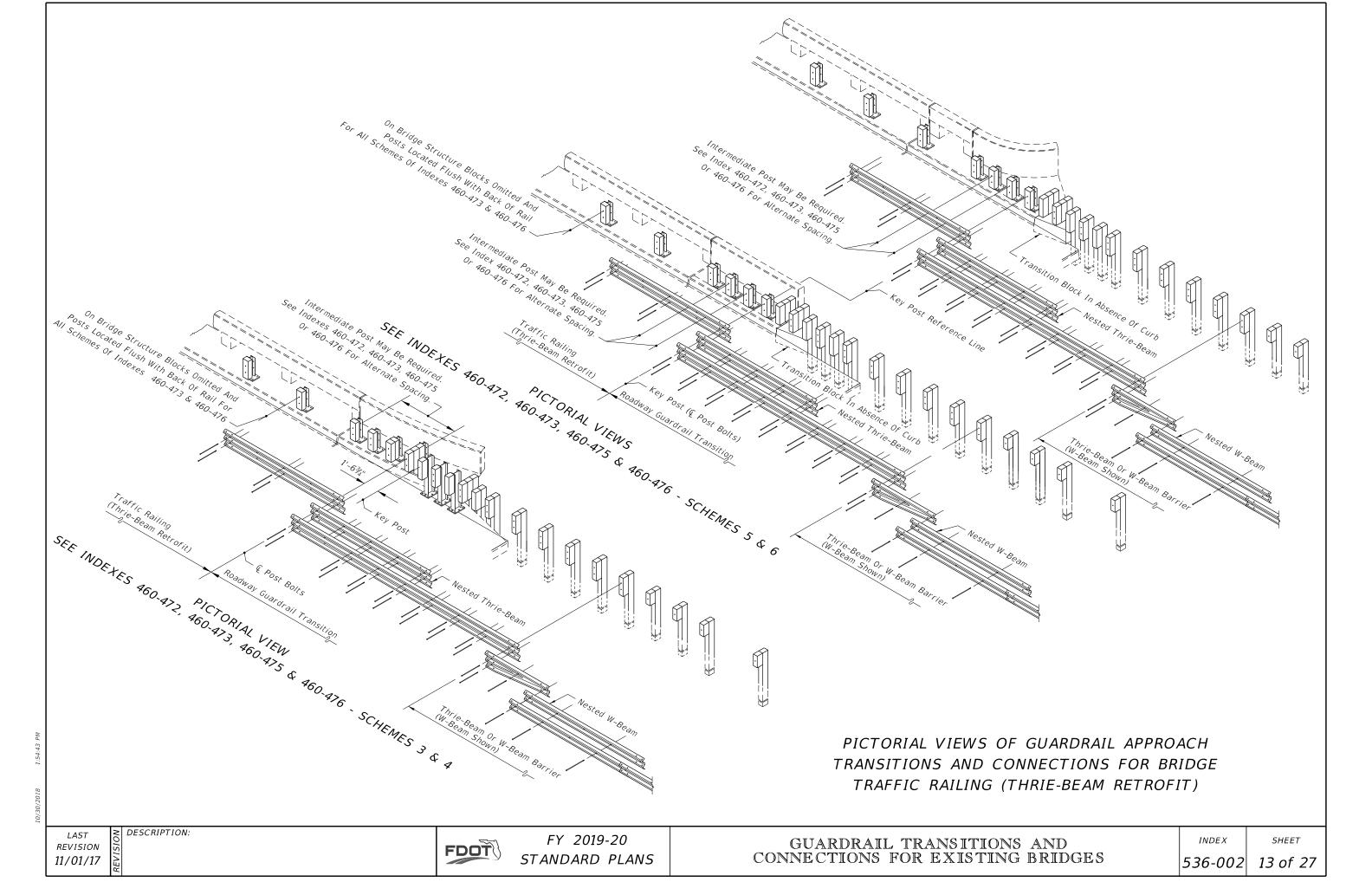
GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES INDEX

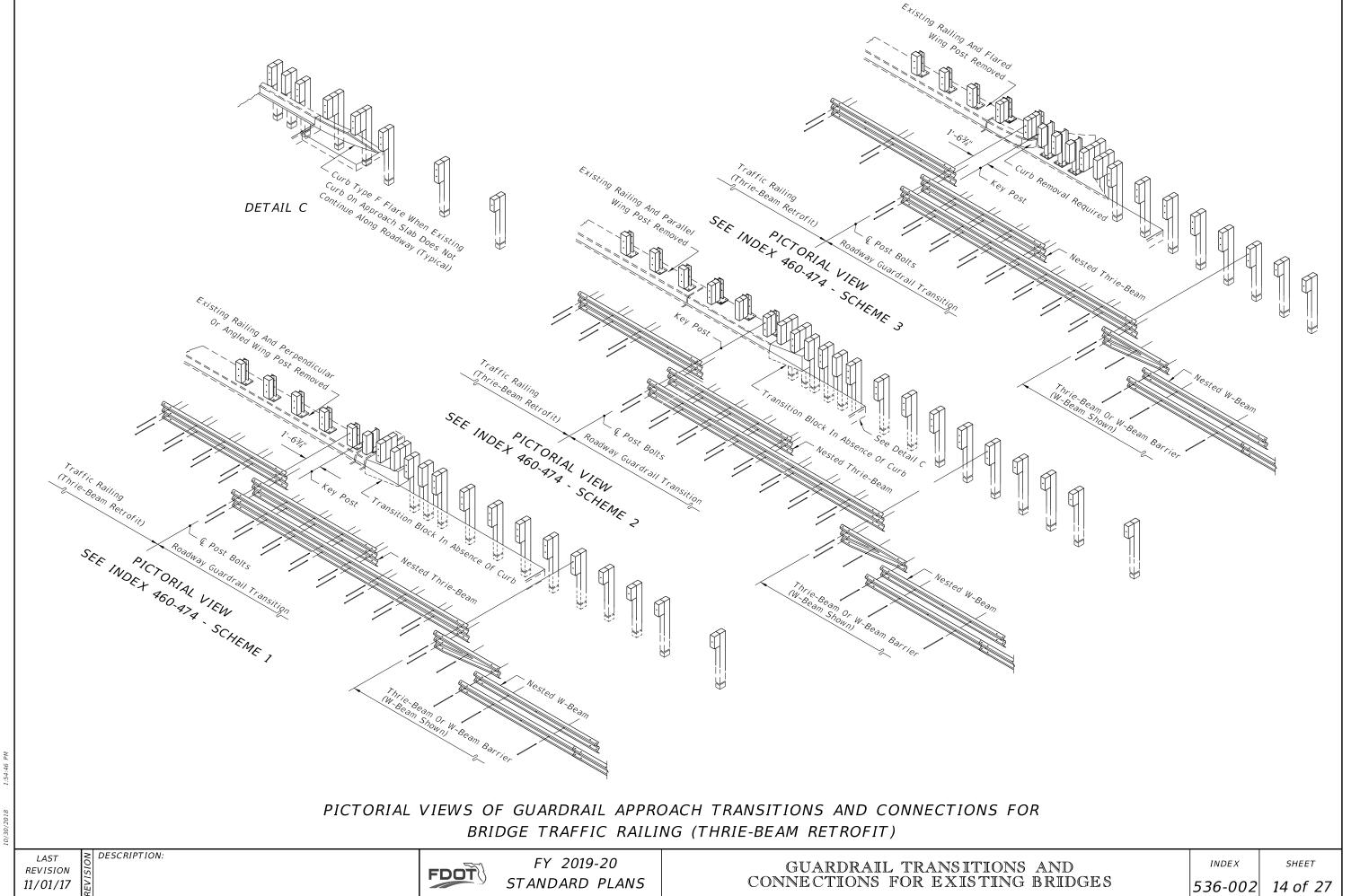
SHEET

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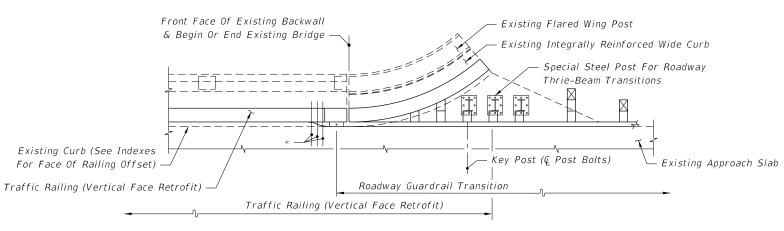


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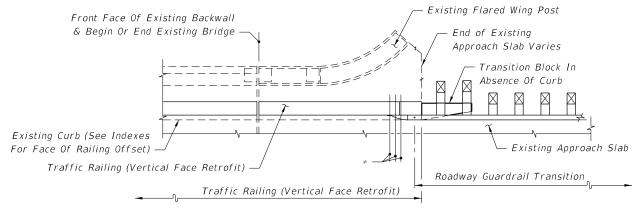
FDOT

STANDARD PLANS

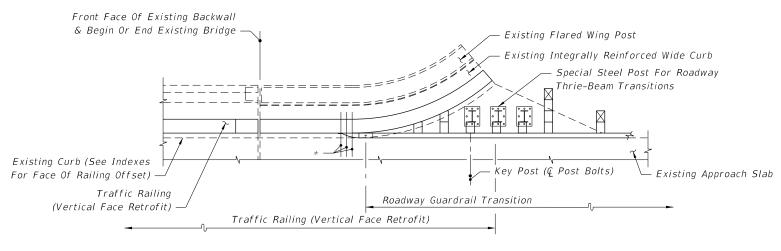
SEE INDEX 521-405 OR 521-482 - SCHEME 2



SEE INDEX 521-405 OR 521-482 - SCHEME 3



SEE INDEX 521-405 OR 521-482 - SCHEME 2



SEE INDEX 521-405 OR 521-482 - SCHEME 3

*21" x 12" x  $\frac{5}{8}$ " Thrie-Beam Terminal Connector Plate (Back-Up Plate), And  $\frac{7}{8}$ "Ø x 12" Long HS Hex Bolts And Nuts (5 Reqd.) With  $2 lac{1}{4}$ " OD Plain Round Washers Under Heads And Nuts

## PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 521-482 SHOWN, INDEX 521-405 SIMILAR)

**REVISION** 11/01/17

DESCRIPTION:

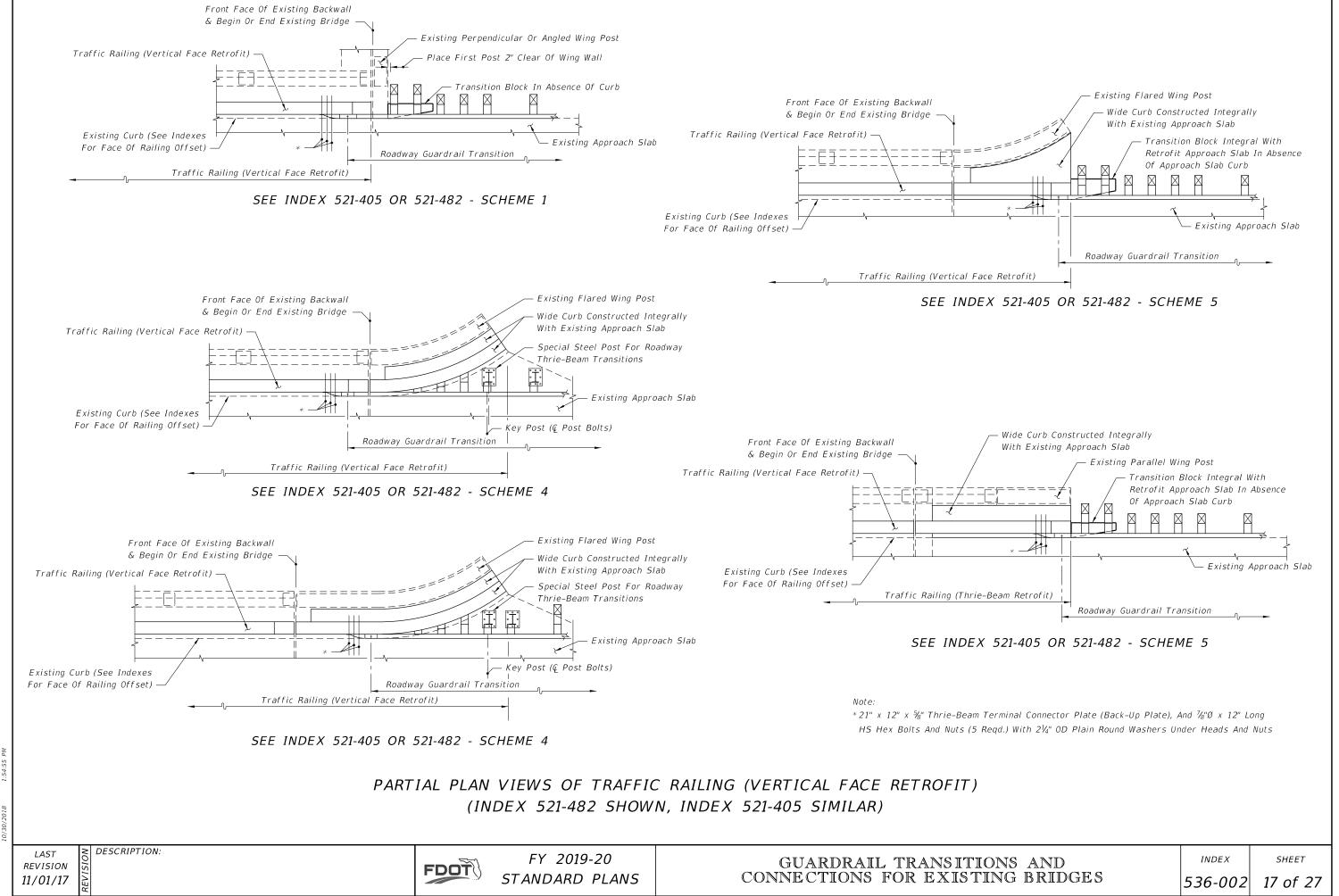


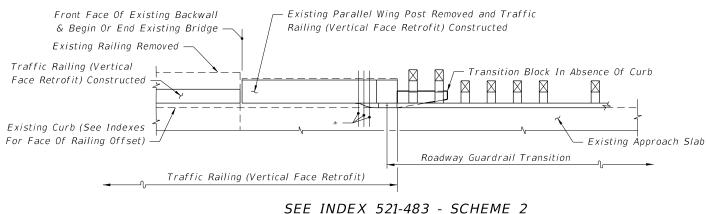
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Front Face Of Existing Backwall

Existing Railing Removed

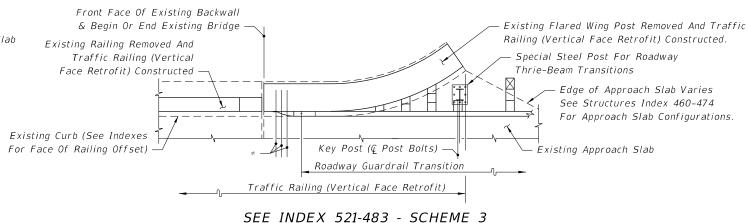
Traffic Railing (Vertical

Face Retrofit) Constructed

Existing Curb (See Indexes For Face Of Railing Offset)

& Begin Or End Existing Bridge -

Traffic Railing (Vertical Face Retrofit)



*21" x 12" x %" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And ½"Ø HS Hex Bolts And Nuts (12" Long For Scheme 1 And Length To Fit For Schemes 2 And 3) (5 Reqd.) With 21/4" OD Plain Round Washers Under Heads And Nuts

## PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)

**REVISION** 11/01/17

**FDOT** 

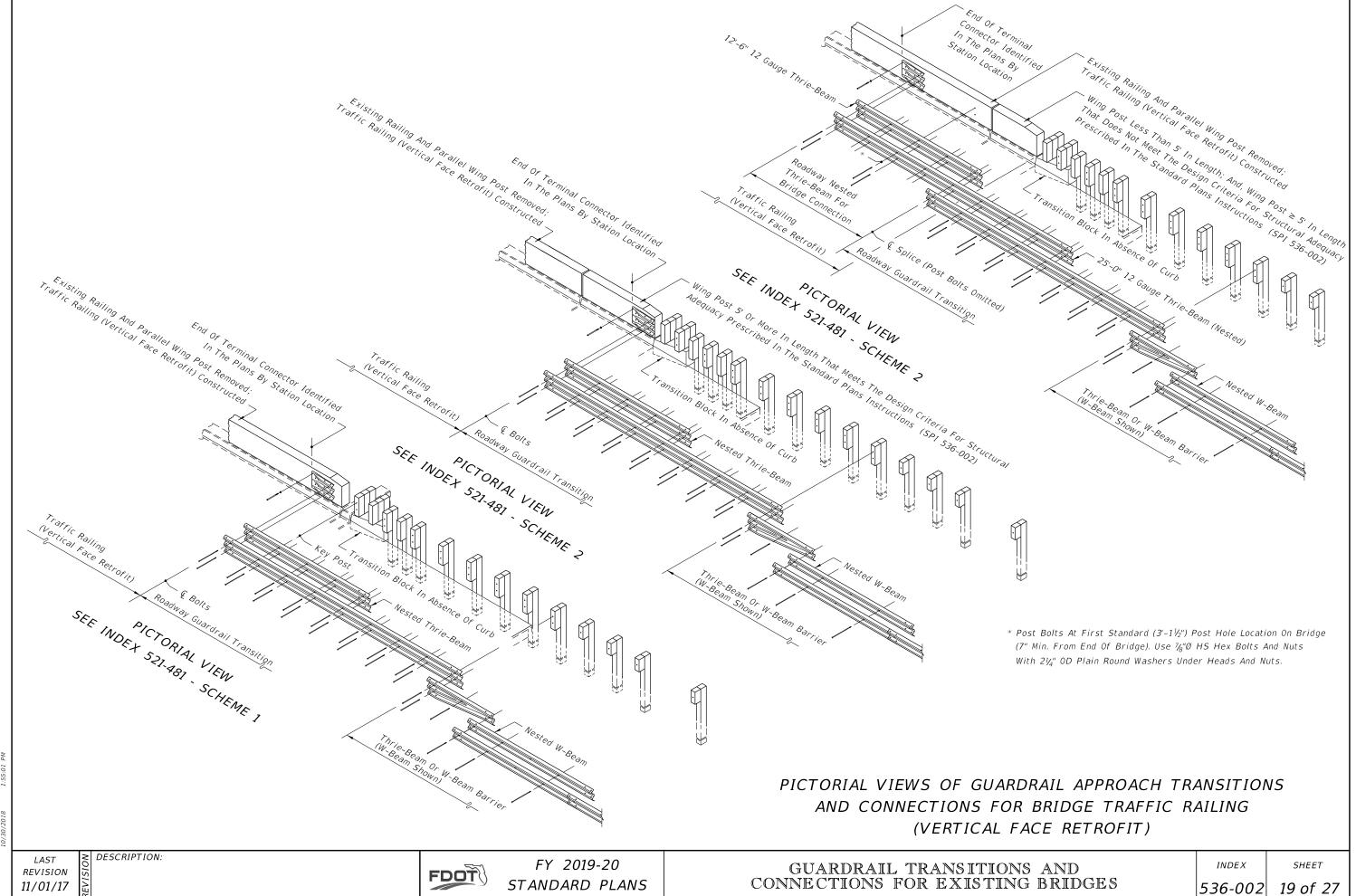
FY 2019-20 STANDARD PLANS

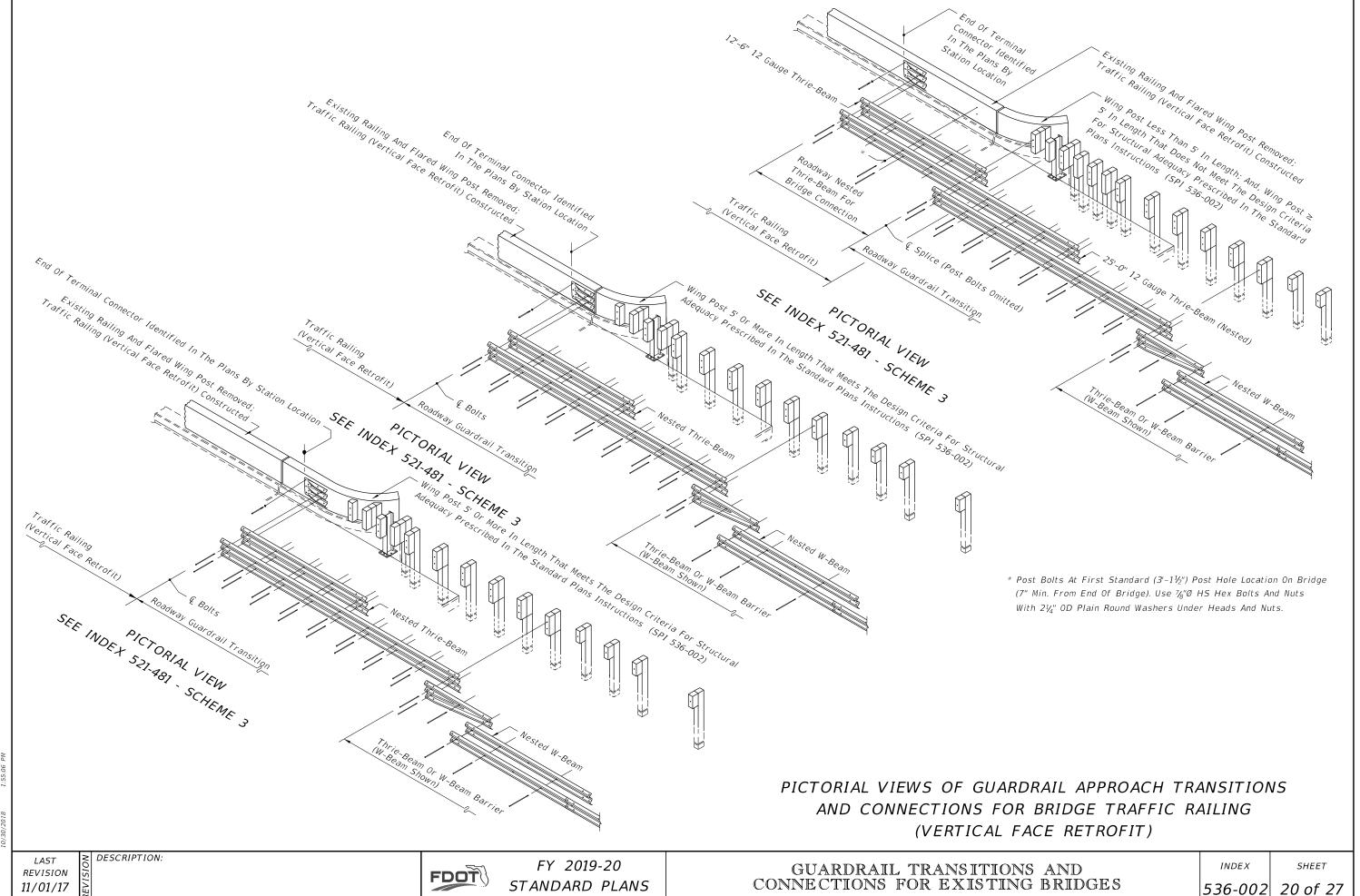
GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

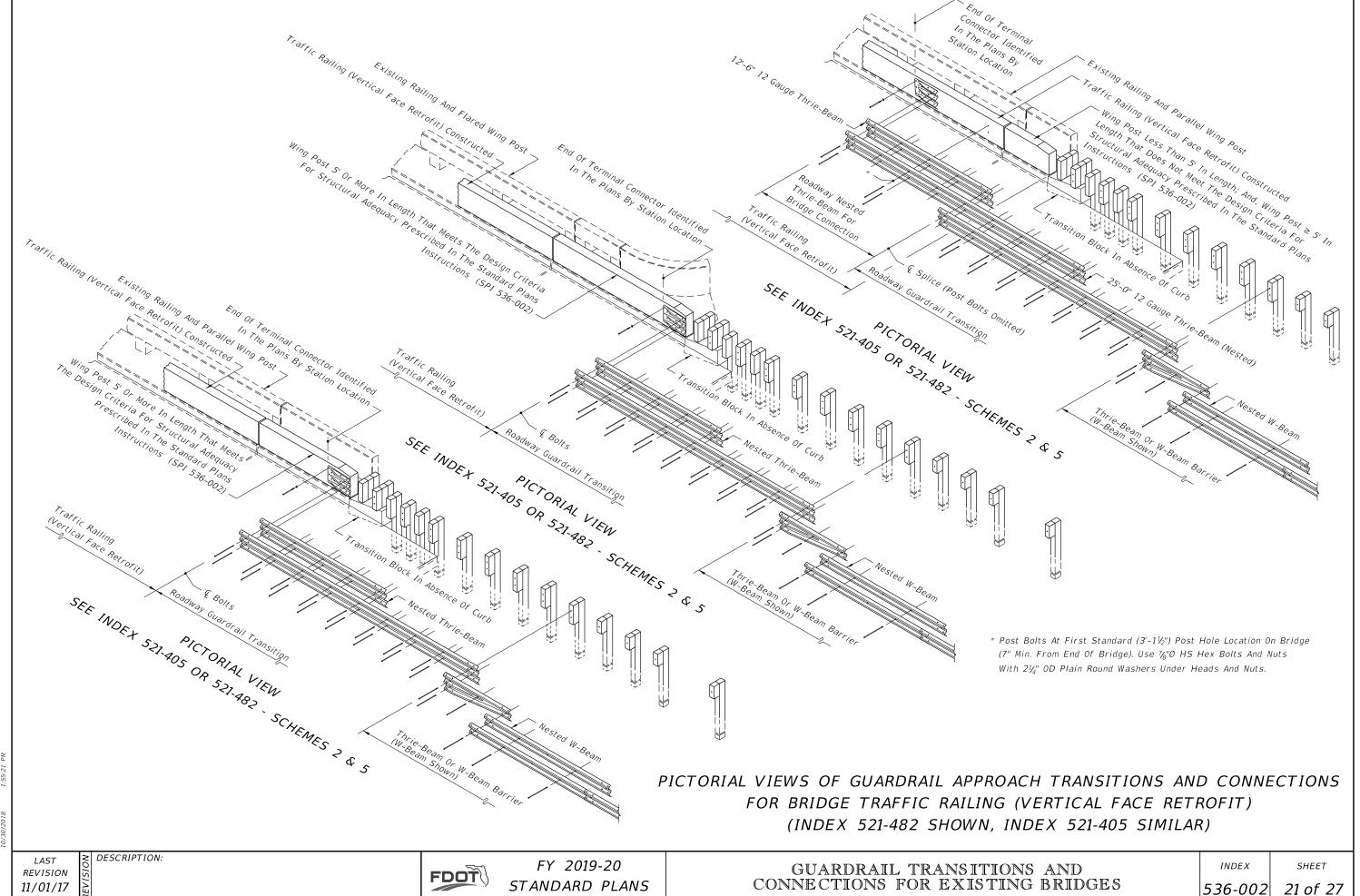
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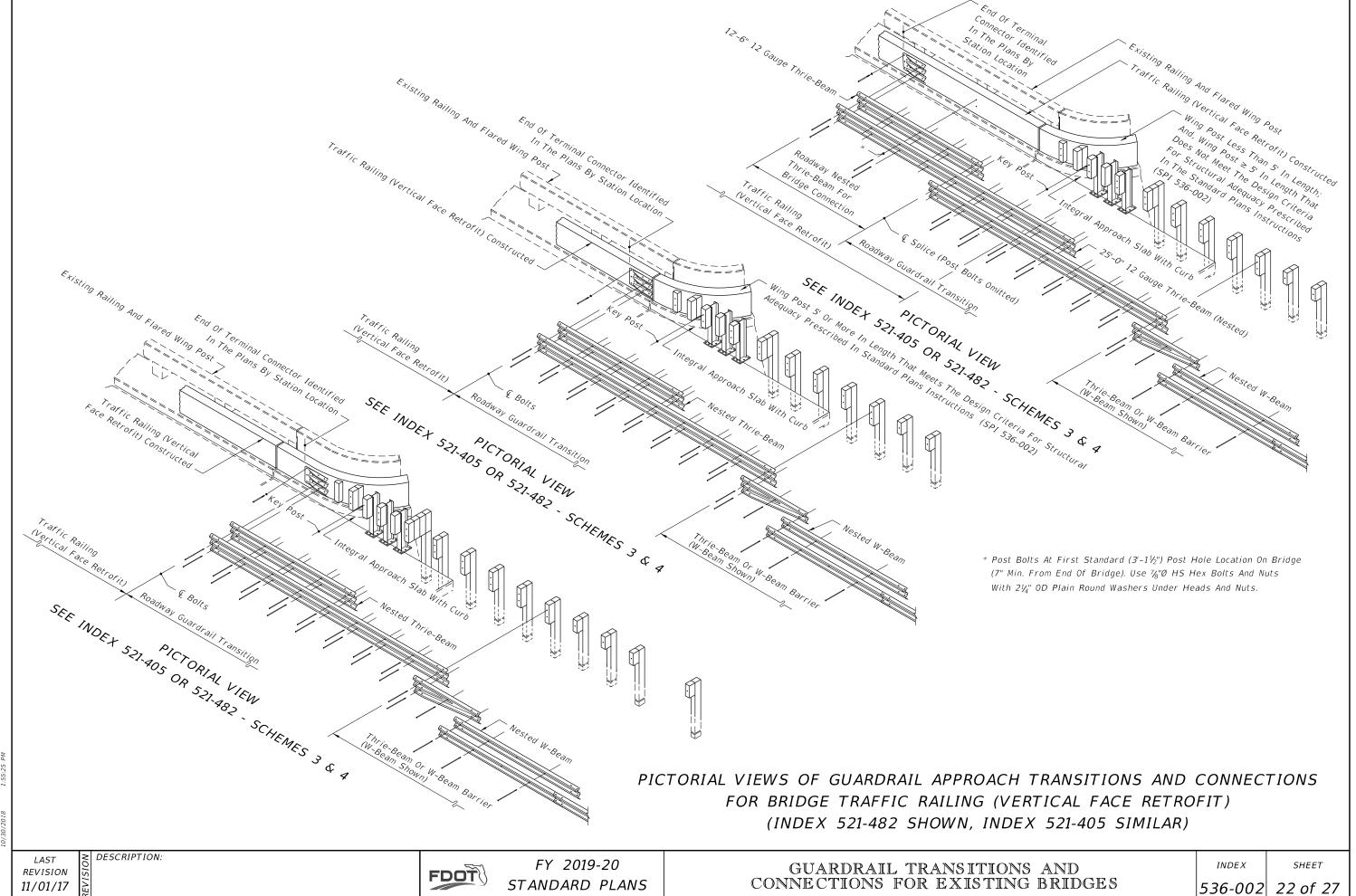
SHEET

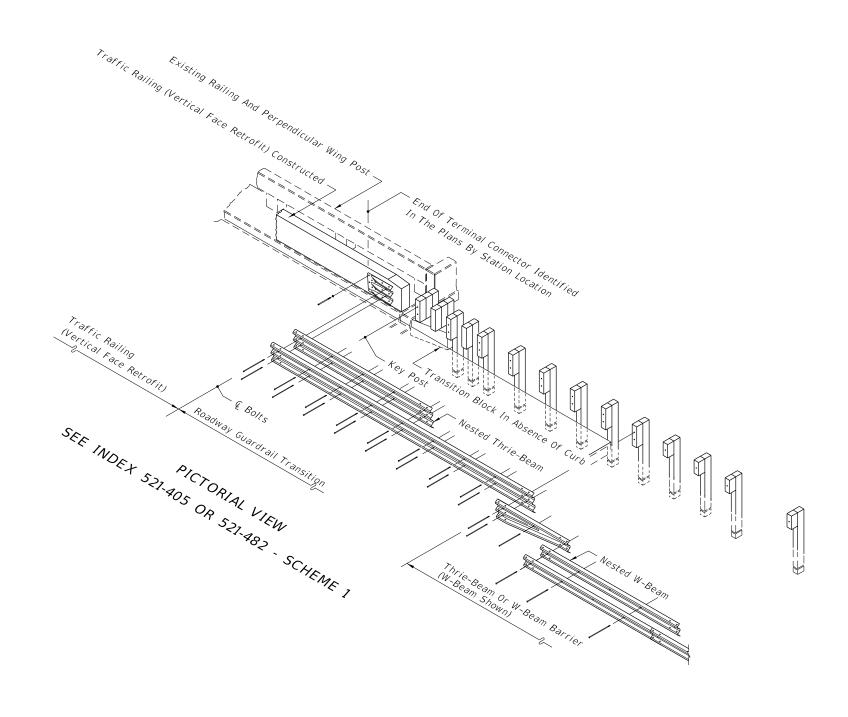
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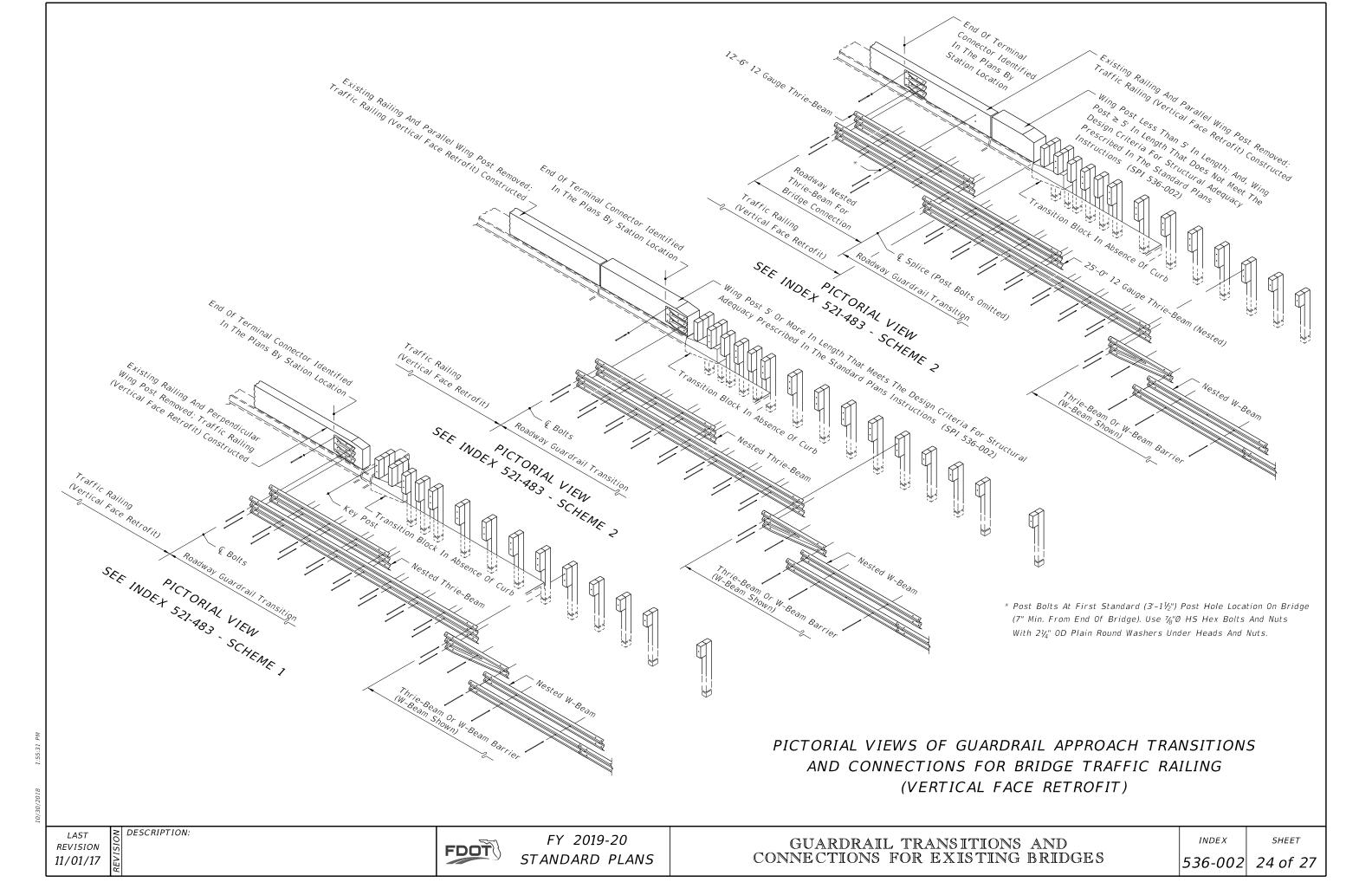


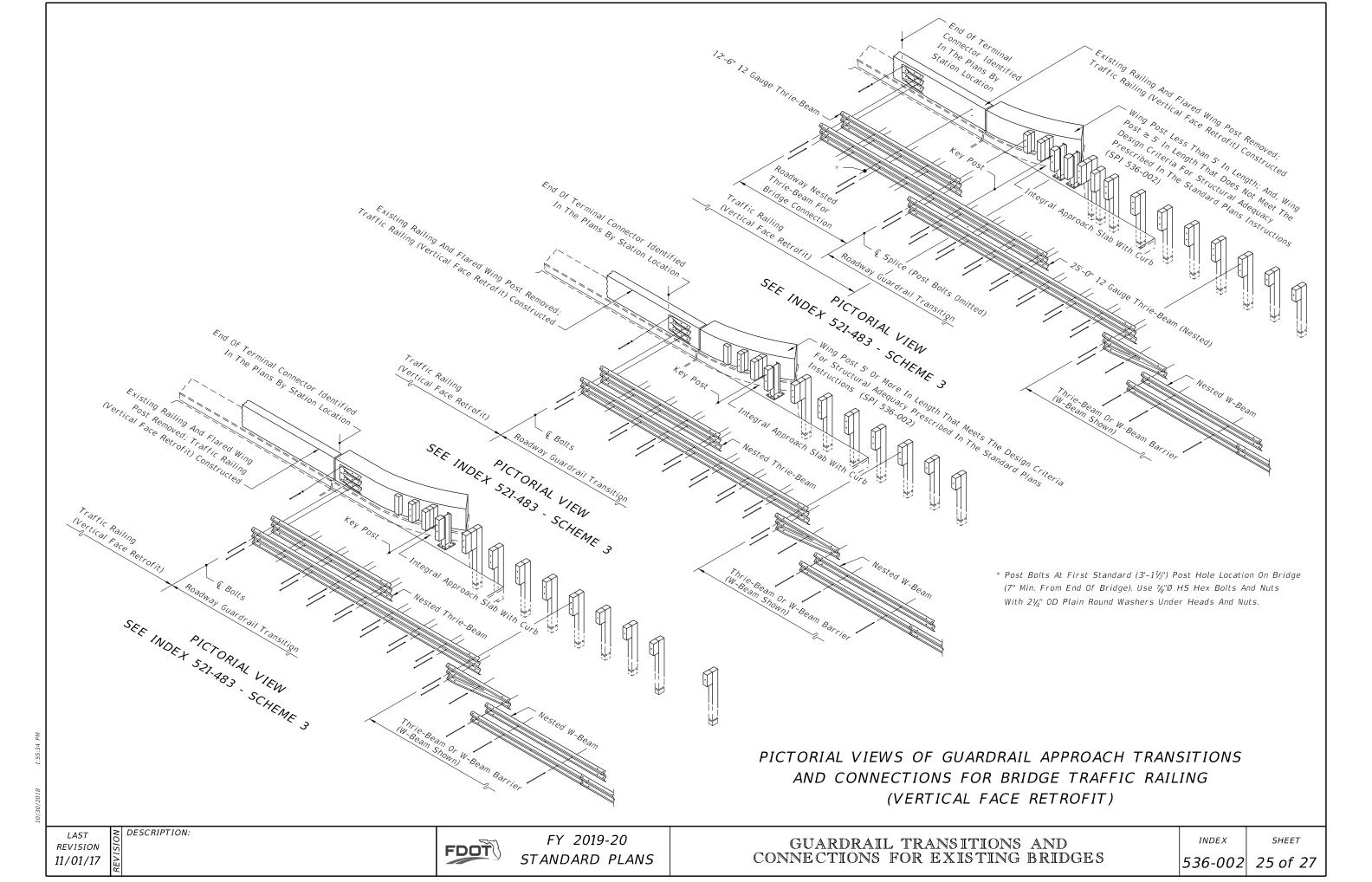


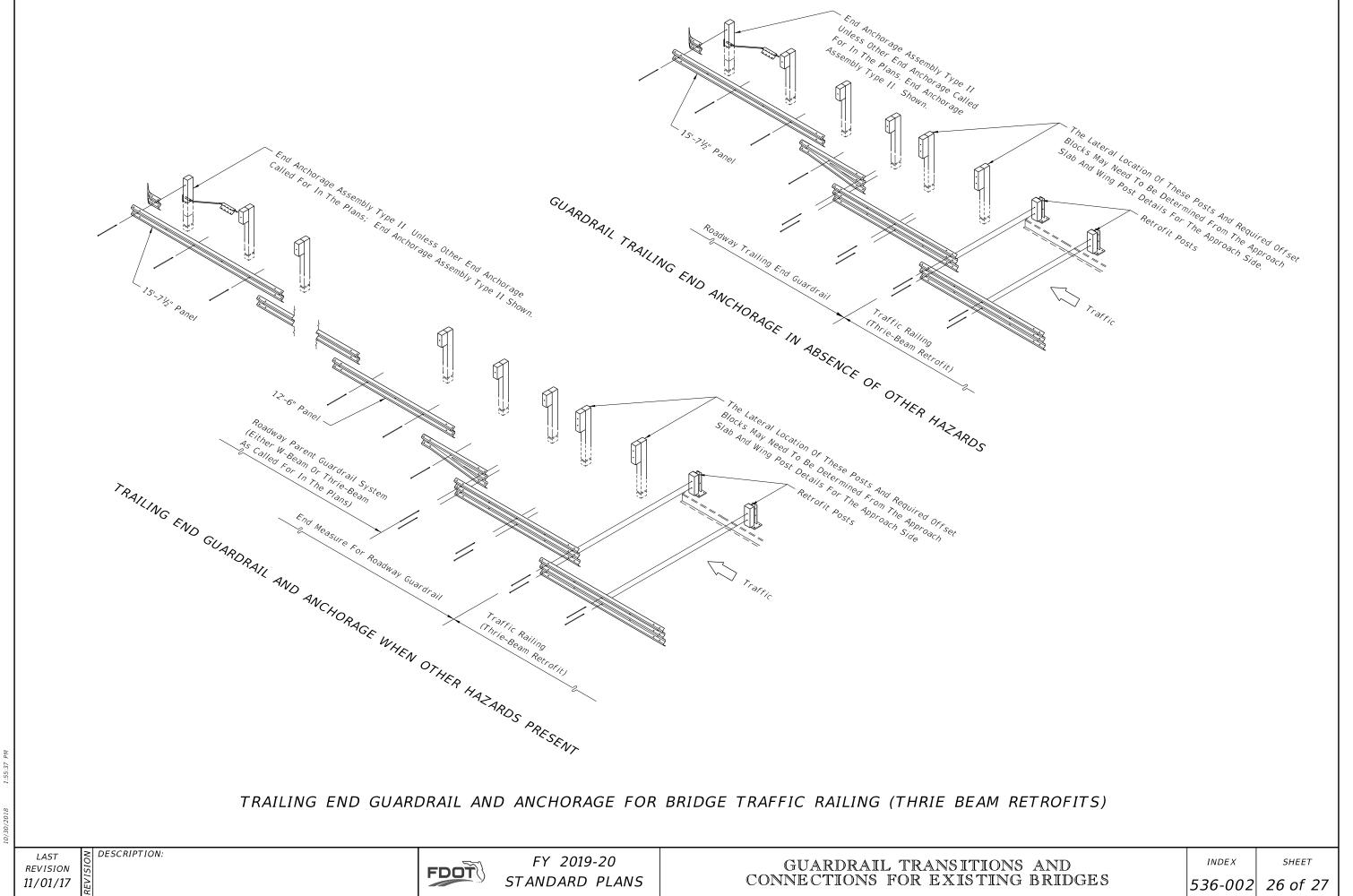


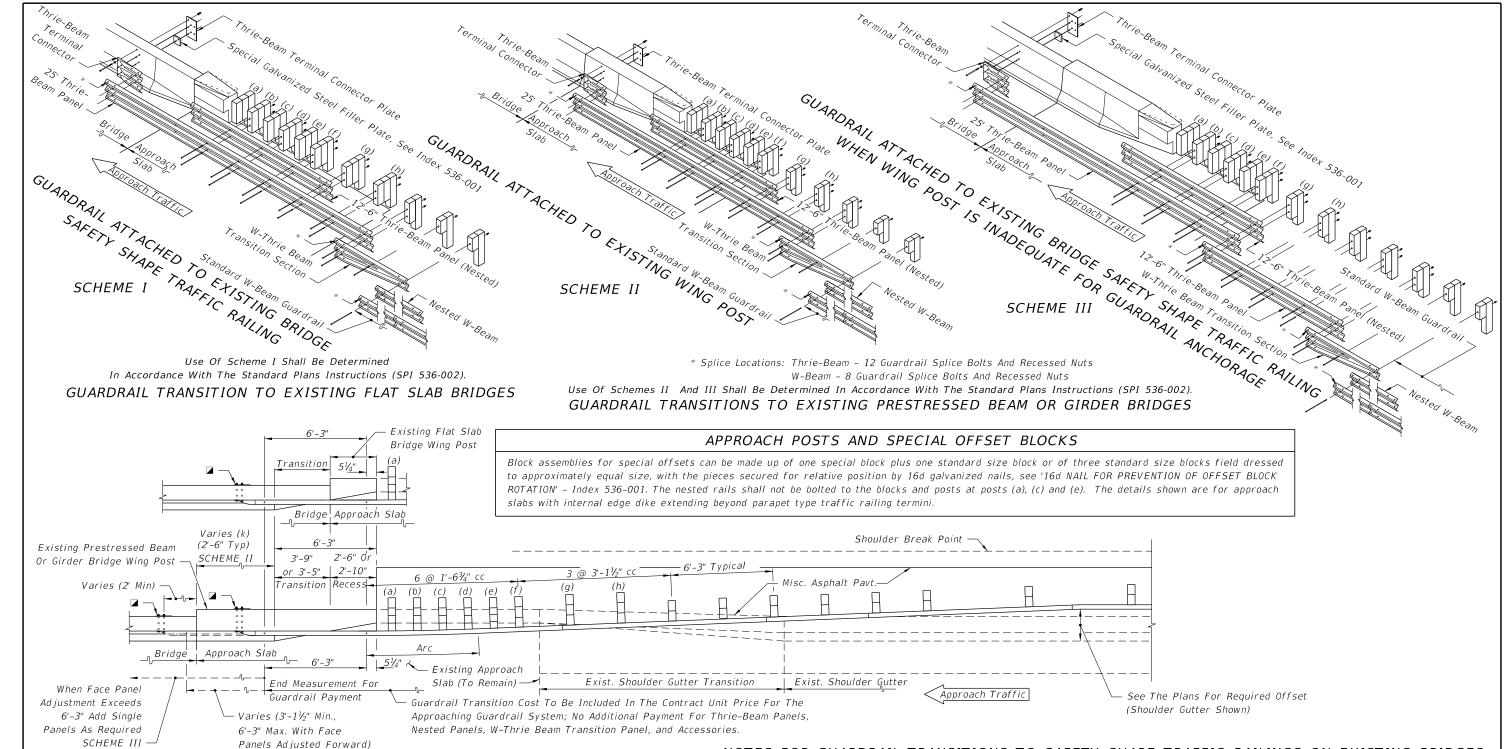
PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 521-482 SHOWN, INDEX 521-405 SIMILAR)

REVISION 11/01/17









☑ 21"x12"x⅙" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And ⅙"Ø x 18" Long [15" Long With 3½" Min. Thread Length For Bridge Safety Shape Railing] HS Hex Bolts And Nuts (5 Reqd.) With 21/4" OD Plain Round Washers Under Heads And Nuts. [When Attaching Guardrail To Existing Wing Posts Or Bridge Rails, Care Should Be Exercised To Avoid Damaging Conduits And Their Utilities That May Be Routed Through Wing Posts Or Bridge Rails. When Conduits And Their Utilities Are Encountered, At Least Five 7/8" HS Hex Bolts Shall Be Installed In Any Of The Seven Holes Provided In The Thrie-Beam Terminal Connector.]

SCHEME III

#### NOTES FOR GUARDRAIL TRANSITIONS TO SAFETY SHAPE TRAFFIC RAILINGS ON EXISTING BRIDGES

- 1. When the guardrail attachment overlays the Bridge Number, Bridge Name or Date on the traffic railing, provide an aluminum sign panel with the obscured information. Attach the sign panel to the face of the traffic railing adjacent to the Thrie-Beam Terminal Connector with 1/4"0 x 1" long concrete screws or expansion anchors at each corner, as approved by the Engineer. The sign panel shall be a minimum  $y_{16}$ " thick and meet the requirements of Specification 700 with a white background and 3" tall black letters and sized appropriately to contain the information required. The cost of the sign panel shall be included in the cost of the Guardrail Bridge Anchorage Assembly
- 2. When retrofitting thrie-beam guardrail to existing wing posts or existing bridge safety shape traffic railing, attachment construction to be paid for under the contract unit price for Guardrail Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate(s) and bolts, nuts and washers.

GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR EXISTING FLAT SLAB, PRESTRESSED BEAM AND GIRDER BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH

**REVISION** 11/01/17

DESCRIPTION:

FY 2019-20 STANDARD PLANS

**PLAN** 

GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

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SHEET

## GENERAL NOTES:

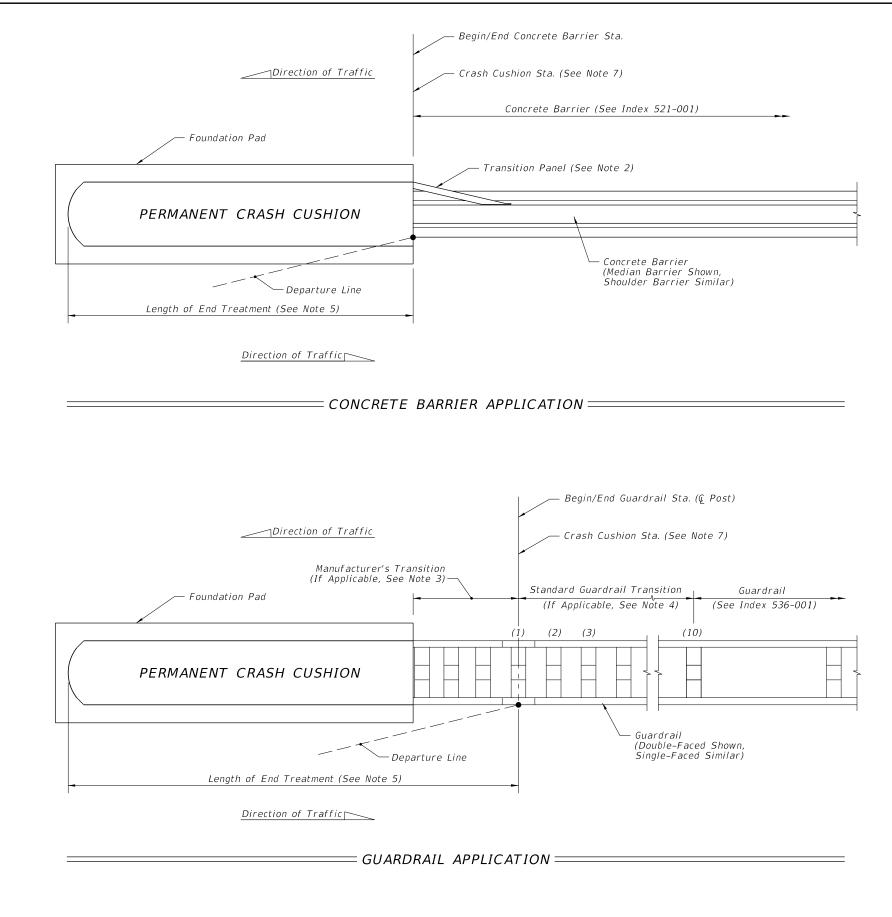
- 1. GENERAL: Work this Index in accordance with Specification 544 and the "Summary of Permanent Crash Cushions" table in the Plans.
- 2. TRANSITION PANEL: Where crash cushions are placed between two-way traffic or adjacent to two-way two-lane traffic, place a Transition Panel from the Concrete Barrier to the Crash Cushion on the downstream side of the barrier end (as shown). Follow the requirements of the APL drawing.
- 3. MANUFACTURER'S TRANSITION: Construct the proprietary guardrail transition only if shown in the applicable APL drawing. See Note 4 below.
- 4. STANDARD GUARDRAIL TRANSITION: If the APL drawing does not provide a guardrail transition to w-beam guardrail, construct the Standard Guardrail Transition segment from thrie-beam to w-beam as shown per Sheet 2. This  $21'-10\frac{1}{2}''$  segment must remain parallel to the roadway.

If the APL drawing does provide a guardrail transition to w-beam guardrail, replace the Standard Guardrail Transition segment with a w-beam guardrail segment at 6'-3" post spacing, except that Post (10) will remain where shown herein if it is located at a guardrail begin or end taper station callout per the Plans. This  $21'-10\frac{1}{2}''$  segment must also remain parallel to the roadway.

5. LENGTH OF END TREATMENT: For Crash Cushions, the Length of End Treatment includes all proprietary elements of the design as shown in the APL drawing, including the manufacturer's transition of guardrail if applicable.

The actual Length of End Treatment varies per Crash Cushion type, but an estimated Length of End Treatment is generally shown in the Plans to provide sufficient space for the Contractor's option of differing Crash Cushion types.

- 6. LENGTH RESTRICTION: In the "Summary of Permanent Crash Cushions" table, if a value is provided in the Length Restriction column, then select a Crash Cushion from the APL which has a Length of End Treatment less than or equal to the value shown. If the table instead shows not applicable (N/A), then Crash Cushion selection is unrestricted regarding length.
- 7. CRASH CUSHION STATION: The Crash Cushion Station point shown herein corresponds to the station provided in the "Summary of Permanent Crash Cushions" table in the Plans.



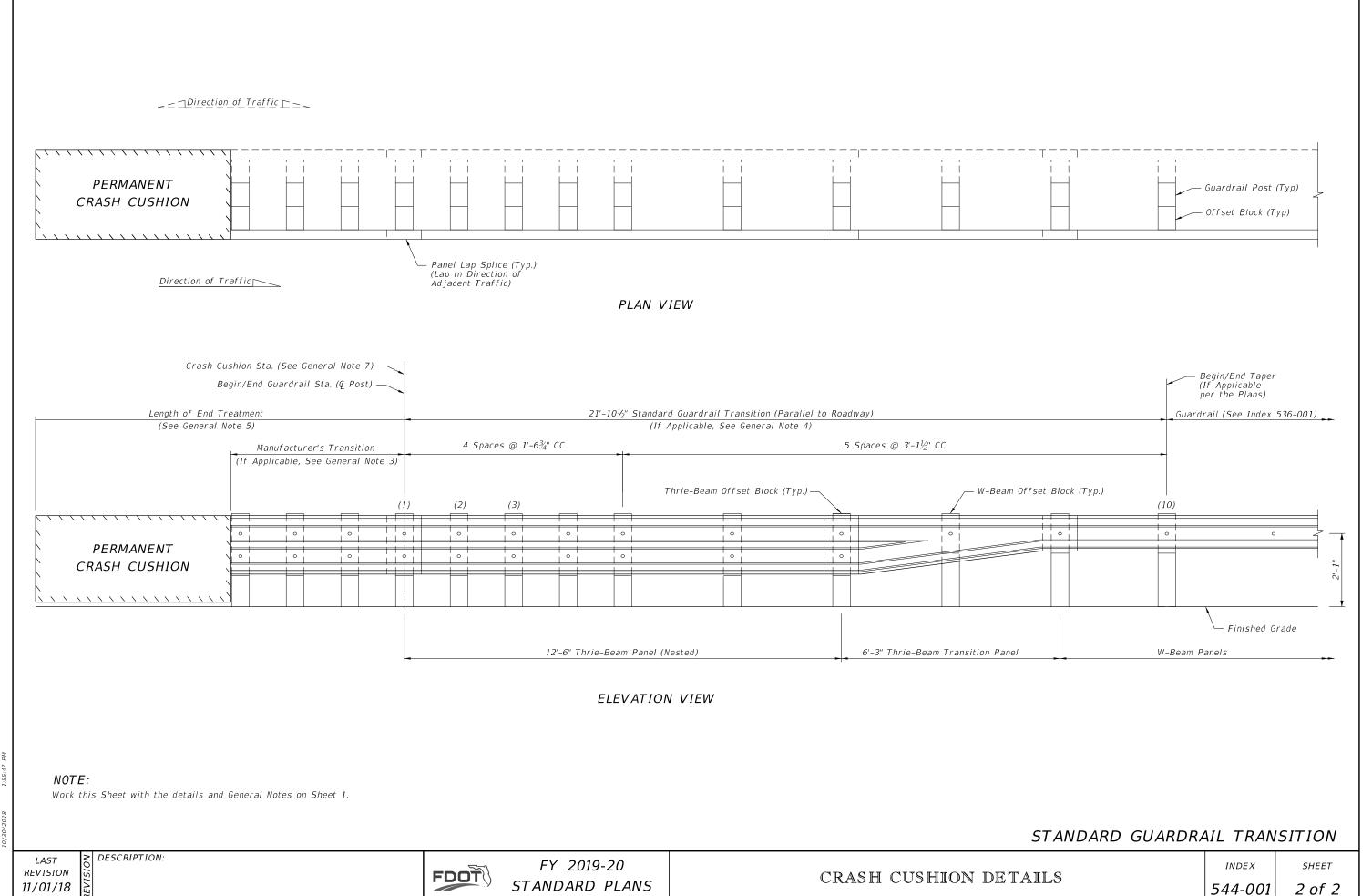
PERMANENT CRASH CUSHION APPLICATIONS

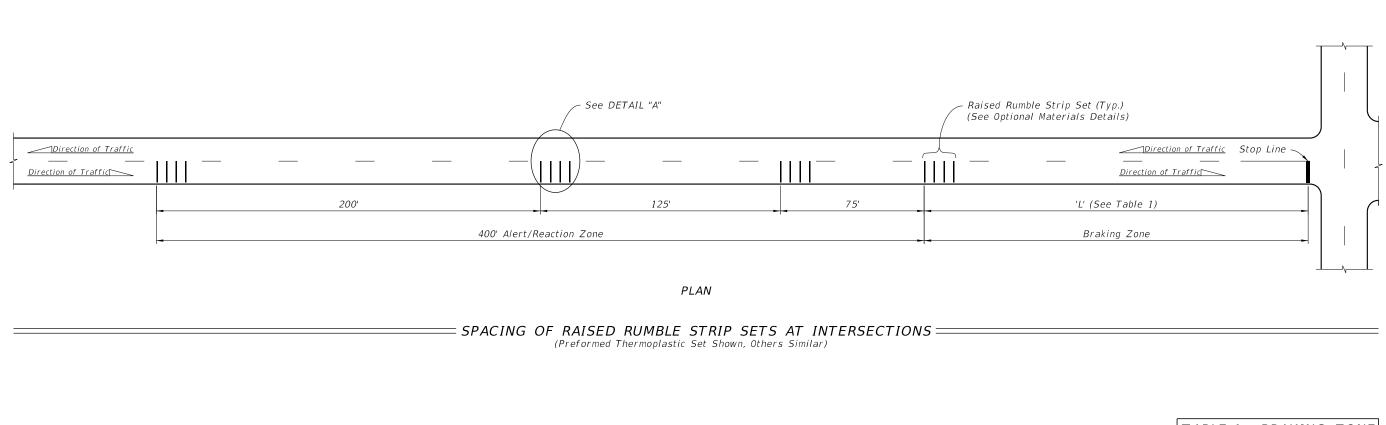
REVISION 11/01/18

DESCRIPTION:



FY 2019-20 STANDARD PLANS





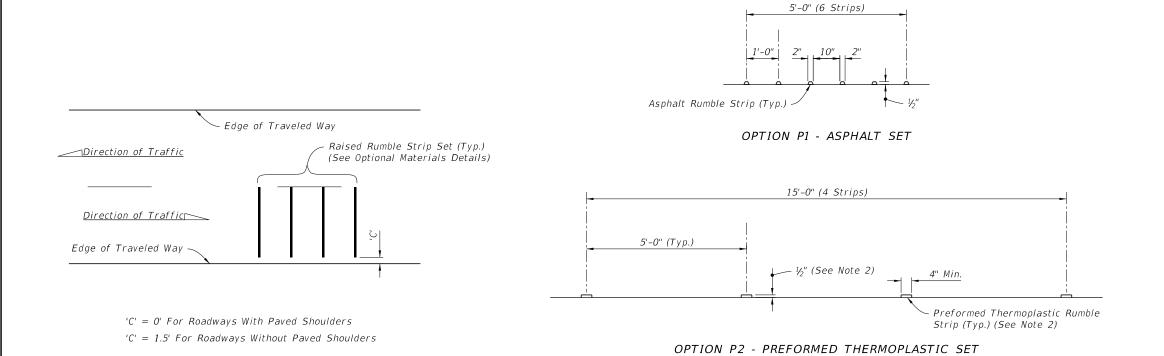


TABLE 1 - BR	AKING ZONE					
Posted	','					
Speed						
(mph)	(Feet)					
≤ 30	150					
35	200					
40	250					
45	300					
50	350					
55	410					
60	470					
65	550					

# NOTES:

- 1. Construct permanent raised rumble strips where shown in the Plans and in accordance with Specification 546.
- 2. Preformed Thermoplastic Set:
- a. Use multiple applications to achieve desired ½" thickness.
- b. Use color white.

PERMANENT RAISED RUMBLE STRIPS

LAST REVISION 04/23/18

FD

DETAIL "A" ===

FY 2019-20 STANDARD PLANS

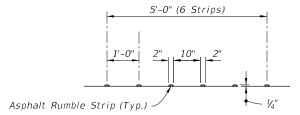
RAISED RUMBLE STRIPS

OPTIONAL MATERIALS DETAILS =

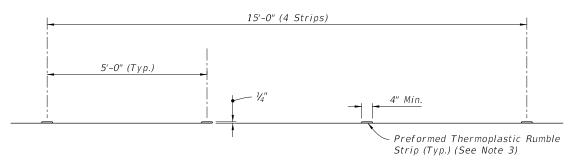
INDEX 546-001

SHEET

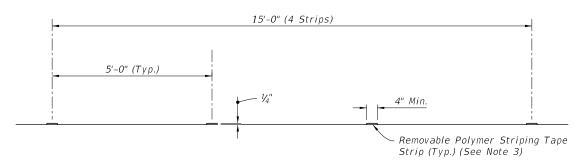
1 of 2



OPTION ST1 - ASPHALT SET



OPTION ST2 - PREFORMED THERMOPLASTIC SET



OPTION ST3 - REMOVABLE POLYMER STRIPING TAPE SET

= OPTIONAL MATERIALS DETAILS ==

# NOTES:

- 1. Construct short-term raised rumble strips where noted in the Plans and in accordance with Specification 546.
- 2. See Sheet 1 for placement and additional details.
- 3. Use color white for Preformed Thermoplastic and Removable Polymer Striping Tape Sets.

SHORT-TERM RAISED RUMBLE STRIPS

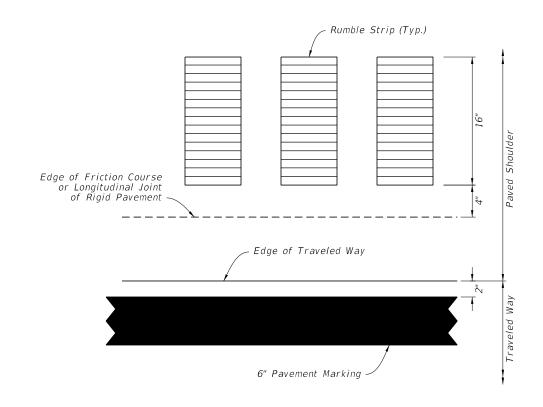
≥ DESCRIPTION: REVISION 04/23/18

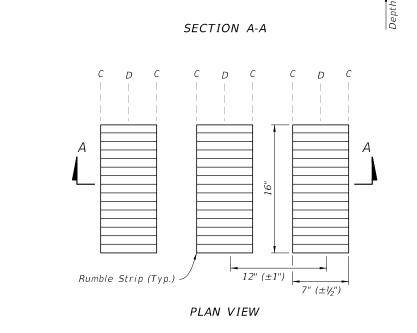
FDOT

RUMBLE STR	IP DEPTH TABLE
LOCATION	DEPTH FROM SURFACE (IN.)
А	0
В	% (±1/16)

# NOTES:

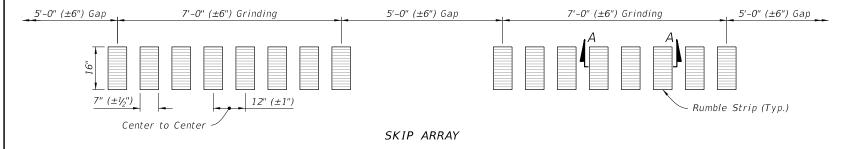
- 1. When friction course extends more than 8" beyond the edge of the traveled way, blade off the extended friction course to the 8" line prior to rumble strip grinding.
- 2. Use the continuous array on both inside and outside shoulders 1,000 feet in advance of bridge ends or back to the gore recovery area for mainline interchange bridges. Use the skip array for all other locations.
- 3. Exclude rumble strips at the following locations:
- A. At mainline tolling areas, terminate rumble strips at the end of the mainline normal section.
- B. At All Electronic Tolling (AET) facilities, terminate rumble strips within 50 feet of the centerline of the overhead gantry.
- C. On outside shoulders of entrance ramp terminals, terminate rumble strips at the point of the physical gore and resume at the end of the acceleration lane taper.
- D. On outside shoulders of exit ramp terminals, terminate rumble strips at the start of the deceleration lane taper and resume at the point of the physical gore.
- E. On approaches to bridges, terminate rumble strips at the approach slab joint.
- F. On either side of median crossover openings, terminate rumble strips within 400 feet.

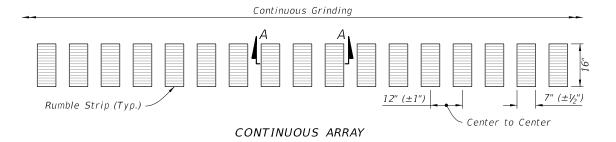




= RUMBLE STRIP PLACEMENT = (Plan View)

:RUMBLE STRIP DETAILS =





= RUMBLE STRIP ARRAY DETAILS =

LIMITED ACCESS ROADWAYS

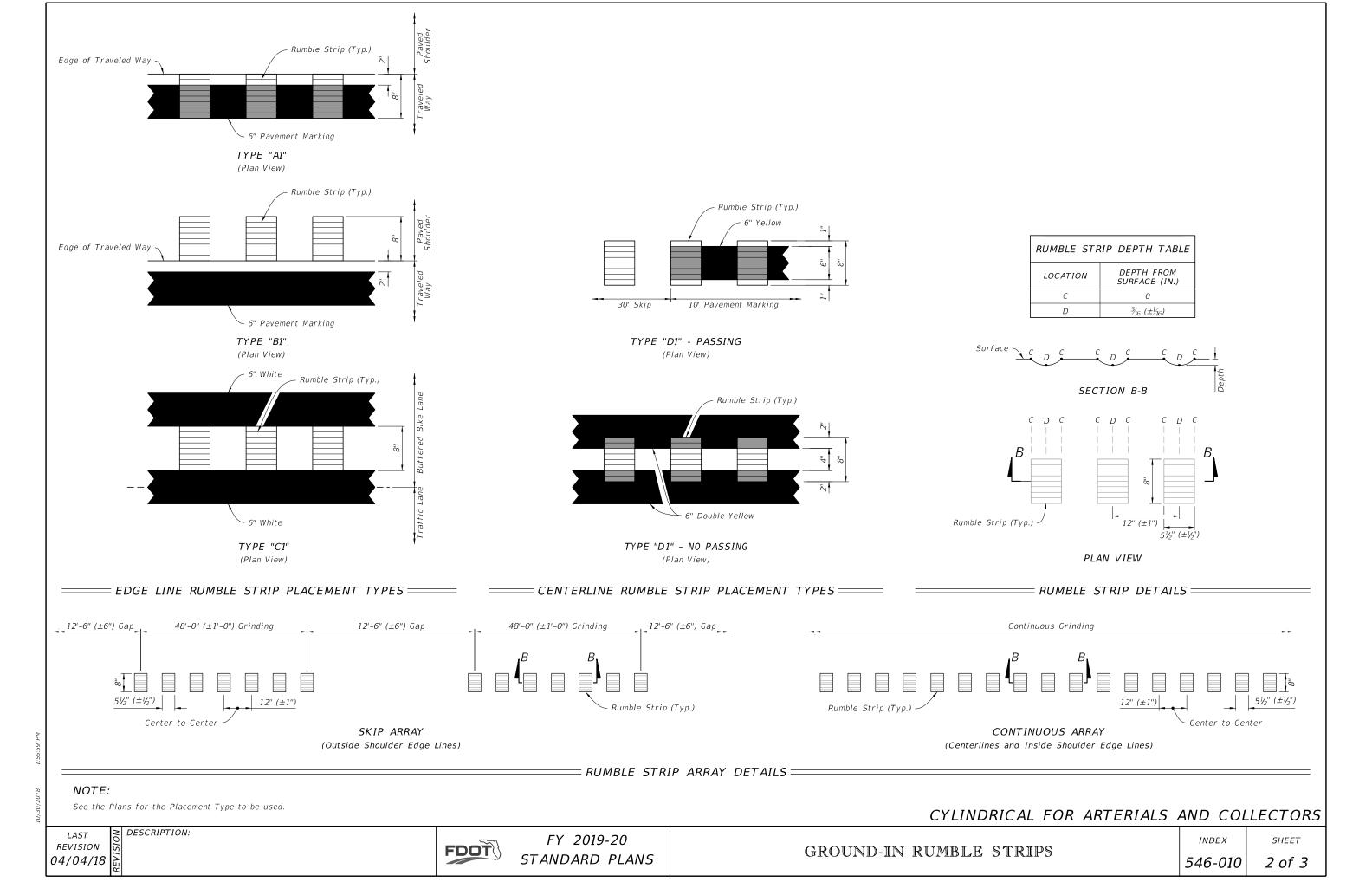
**REVISION** 04/04/18

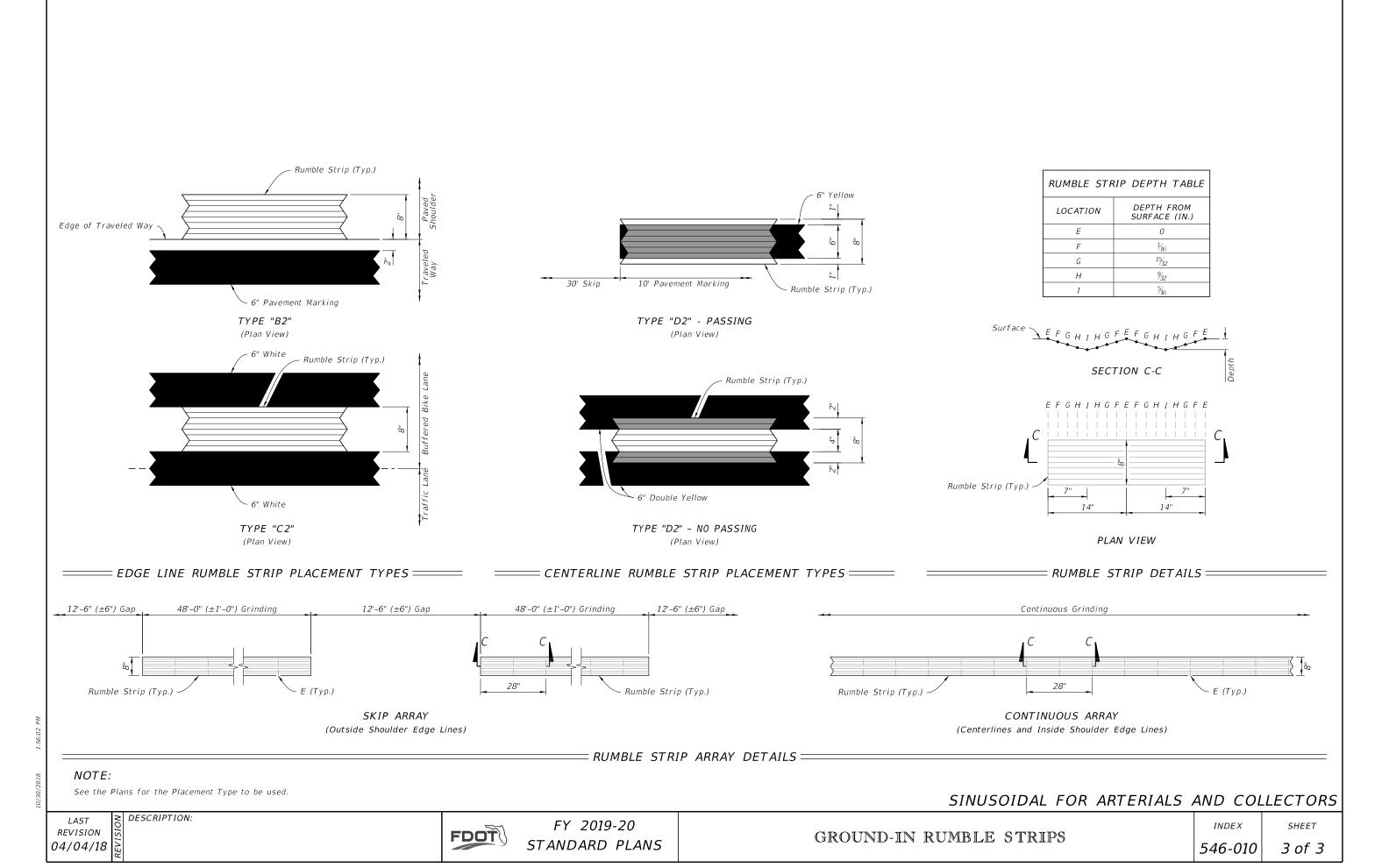
DESCRIPTION:

FY 2019-20 STANDARD PLANS

INDEX 546-010

SHEET 1 of 3





#### SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

#### MATERIALS

1. See Specification Section 548 for material requirements.

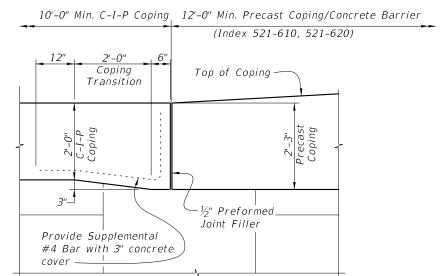
#### CONSTRUCTION:

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. If required, locate manholes and drop inlets as shown on wall elevations.
- 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 8. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.
- 9. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 10. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 11. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 12. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- 13. The top of the leveling pad or footing will be 2'-0" minimum below final ground line.
- 14. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 15. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 16. Work this Index with Index 521-600 thru 521-650.

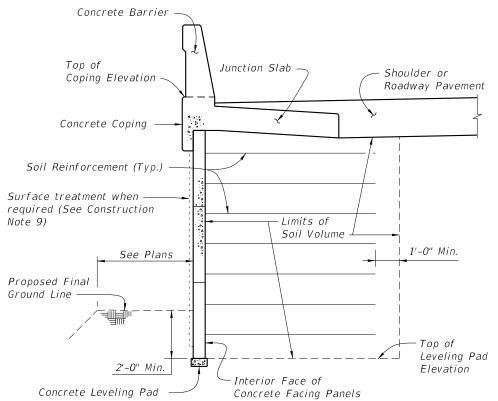
DESCRIPTION:

#### SHOP DRAWINGS:

See Specification Section 548 for shop drawing requirements.







TYPICAL MSE RETAINING WALL SECTION
WITH A CONCRETE BARRIER
(Showing Limits of the Reinforced Soil Volume)

		FD	OT MSE	RETAINI	NG WALL	CLASSI	FICATION TAI	BLE					
Applicable	Durability Requirements (Carbon-Steel Reinforcing)			Durability Requirements (FRP Reinforcing)			Soil	Other Allowable FDOT Wall Types					
FDOT Wall	Concrete	Concrete	Pozzolan	Concrete	Concrete	Pozzolan	Reinforcement						
Type *	Cover	Class	Additions?	Cover	Class	Additions?	Type	2A	2B	2C	2D	2E	2F
	(in.)	for Panels	**	(in.)	for Panels	**							
Type 2A	2	II	No	1.5	II	No	Metal		/	/	/	/	/
Type 2B	2	IV	No	1.5	IV	No	Metal			/	/	/	1
Type 2C	3	IV	No	1.5	IV	No	Metal				/	/	/
Type 2D	3	IV	Yes	2	IV	No	Metal					/	/
Type 2E	3	IV	No	2	IV	No	Plastic						/
Type 2F	3	IV	Yes	2	IV	No	Plastic						

- * See Data Table in Contract Plans.
- ** Silica fume, metakaolin or ultrafine fly ash.

GENERAL NOTES AND DETAILS

LAST REVISION 11/01/18

FDOT

FY 2019-20 STANDARD PLANS

#### DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and FDOT Structures Design Guidelines Section 3.13.2.
- 2. It is the responsibility of the Engineer to determine that the factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer.
- 4. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.

#### SOIL PARAMETERS:

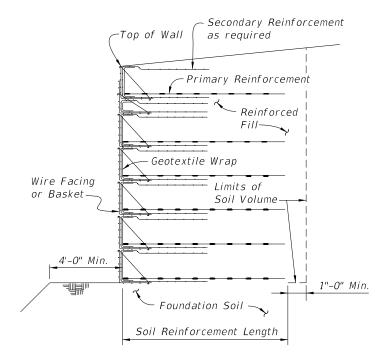
1. See wall control drawings for soil characteristics of foundation material to be used in the design of the wall system. The Contractor must provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site. Provide the values of unit weight, cohesion and internal friction angle in the Shop Drawings.

#### MATERIALS:

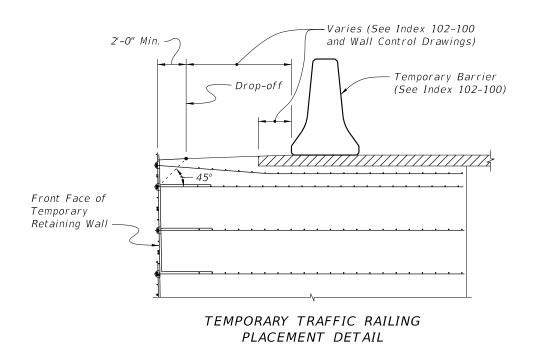
- 1. Provide soil reinforcement in accordance with Specification Section 548.
- 2. For additional material notes, see Wall Company General Notes.

#### CONSTRUCTION:

- 1. Walls must be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. Refer to Plan and Elevation sheets of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 4. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor must notify the Engineer to determine what course of action should be taken.
- 5. The Contractor is responsible for gradually deflecting upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.



TYPICAL RETAINING WALL SECTION (Showing Limits of the Reinforced Soil Volume)



GENERAL NOTES AND DETAILS

10/30/2010

LAST REVISION 11/01/17

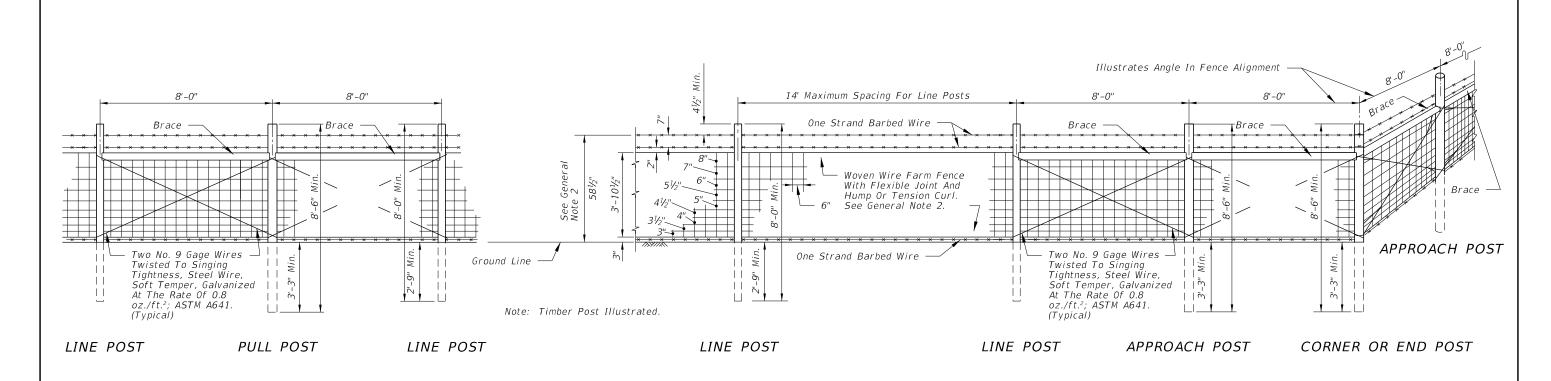


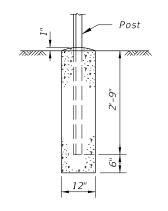
- 2. Fabric shall be woven wire, either galvanized steel, meeting the requirements of ASTM A116, No. 9 Grade 60, Design Number 1047-6-9, with Class 3 zinc coating; No. 12 ½ Grade 175, Design Number 1047-6-12 ½, with a 10 ½ gage top and bottom wire and with Class 3 zinc coating; or aluminum coated steel, meeting the requirements of ASTM A584, No. 9 Farm, Design Number 1047-6-9, with a minimum coating weight of 0.40 oz./ft?. For additional information see payment note below.
- 3. Fence shall be installed with wire side to private property except on horizontal curves greater than 3° the fence shall be installed so as to pull against all posts.
- 4. Posts may be either timber, steel, recycled plastic or concrete. Unless a specific post material is called for in the plans, the Contractor may elect to use either a single material or a combination of timber, steel, recycled plastic or concrete materials, but must comply with the electrical grounding requirements in Specifications 550. Line posts of one material may be used with corner, pull and end post assemblies of a different material. Line posts of only one optional material and pull post assemblies of only one optional material will be permitted between corner and end post assemblies. Within individual corner and end post assemblies only one optional material will be permitted.
- 5. Timber posts shall meet the material requirements of Specification 954. Timber line posts are to be minimum 4" diameter. Timber corner, pull, approach and end posts are to be a minimum 5" diameter. Timber braces are to be minimum 4" diameter.
  - (A) Staples for line posts to be  $1\frac{1}{4}$ " minimum length; for approach, corner and pull posts  $1\frac{1}{2}$ " minimum length. At approach, corner and pull posts, staple every line wire. At line posts, staple every line wire in top half and alternate line wires in bottom half. Staples shall be driven diagonally across the line wire with the points in separate grains.
  - (B) Connections between timber posts and braces to be provided by dowels as shown in fastener details.
  - (C) Wire to be wrapped and tied, as shown in the splice details, at the following locations:
    - (a) All end posts, (b) Corner post, including the assemblies at vertical breaks of 15° or more and (c) Pull posts where the wire is not spliced and pulled through the assembly; see General Note 18.
- 6. Steel posts and braces shall be standard steel posts, galvanized at the rate of 2 oz./ft.², together with necessary hardware and wire clamps and meeting the following requirements:
  - (A) Line posts: 8' long; 1.33 lbs./ft.; roll formed studding; anchor plate attached, ASTM A702 (18 in.2).
  - (B) Approach posts: 2½"x2½"x½" angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
  - (C) Pull, end and corner posts:  $2\frac{1}{2}$ "x $2\frac{1}{2}$ "x $\frac{1}{4}$ " angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
  - (D) Braces: 2"x2"x1/4" angles with necessary hardware and fabricated for attaching to post.
  - (E) The pull, corner, approach and end posts are to be set in concrete as per detail. (Also see General
- 7. Recycled plastic posts shall meet the following material requirements: Line posts shall have a minimum section of 4" round or 4" square. Plastic posts shall not be used as corner, pull, end or approach posts unless such use is specifically detailed in the plans. The straightness of the post shall comply with Specification 954 for timber post. The flexural strength shall meet the requirements of the latest edition of the Southern Pine Inspection Bureau's Standard Grading Rules for Southern Pine Lumber for No. 2SR Stress Rated Grade Timber. Plastic posts can be set by either digging and tamped backfill or by driving into full depth preformed holes 1/4" to 1/5" smaller than cross section of
  - Staples for fabric and barbed wire connection to plastic line posts shall be the same size, count and location as that for timber posts.
- 8. The Contractor, at his option, may use any suitable precast or prestressed concrete posts; however, approval by the Engineer, of posts not shown on this index, will be required prior to construction of the fence. Precast posts shall be Class I concrete. Prestressed posts shall be Class III concrete. Lengths of concrete post to be as indicated for timber posts.
- 9. Aluminum post, braces and accessory framing hardware shall not be used unless the plans specifically detail their application or the Engineer specifically approves their incorporation in fence construction or repair. Aluminum framed gates are permitted as described in General Note 19.

- 10. The woven wire shall be attached to steel and concrete posts by a minimum of five tie wires. The single wire ties shall be applied to the top, bottom and three intermittent line wires. The ends of each tie wire shall have a minimum of two tight turns around the line wire. Tie wires shall be steel wire not less than 0.120" diameter, zinc coating Class 3, soft temper, in accordance with ASTM A641.
- 11. Steel Barbed Wire can be either of the following types:
  - Type 1: This type shall conform to the requirements of ASTM A121, with two strands of 12½ gage wire; four-point barbs, wire size 14 gage, twisted around both line wires; and, Class 3 coating, Design No. 12-4-5-14R.
  - Type IIA: This type same as Type I except the two strand wires are twisted in alternating directions between consecutive barbs.
  - Type IIB: This type shall conform to the requirements of ASTM A121 with two strands of 15 ½ gage high tensile wire; four-point barbs, wire size 16 ½ gage twisted around both line wires; and Class 3 coating, Design No. 15-4-5-16R.

Aluminum Barbed Wire shall be fabricated of two strands of 0.110-inch wire with 0.08-inch diameter four-point barbs spaced at approximately 5½", and at a maximum spacing of 6". The wire for the strands and for the barbs shall be of ASTM B211M Alloy 5052-H38 or equal.

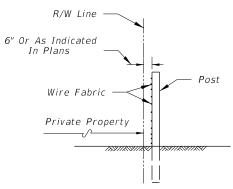
- 12. The woven wire shall be stretched only until one-half the tension curl has been pulled out of the line wires.
- 13. Posts to be set by driving or digging. If by digging, the posts shall be set at the center of the hole and the soil tamped securely on all sides.
- 14. Longer posts than those indicated above may be required by the plans or for deeper installations.
- 15. Concrete bases for angular steel posts (pull, corner, end and approach) shall be Class NS in accordance with Specification 347. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 16. Pull post assemblies shall be installed at approximately 330' centers except that this maximum interval may be reduced by the Engineer on curves where the radius is less than 3°.
- 17. Corner post assemblies are to be installed at all horizontal and vertical breaks in fence of 15° or more.
- 18. A maximum length of 1320' of wire may be installed as a unit. For pulls through a pull post assembly the fabric shall be spliced by crimping sleeves only. Pulls through a corner post assembly will not be permitted
- 19. Unless otherwise called for in the plans gates shall be commercially available metal swing gates assembled and installed in accordance with the manufacturer's specifications as approved by the Engineer. Chain link swing gates in accordance with Index 550-002 may be substituted for metal swing gates as approved by the Engineer. Gate size is full opening width whether single leaf or double leaves. Payment for gates shall include the gate, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 20. For construction purposes, assemblies are defined as follows: End post assemblies shall consist of: one end post, one approach post, two braces, four diagonal tension wires and all necessary fittings and hardware. Pull post assemblies shall consist of: one pull post, two braces, four diagonal tension wires and necessary fittings and hardware. Corner post assemblies shall consist of: one corner post, two approach posts, four braces, eight diagonal tension wires and all necessary fittings and hardware.
- 21. All posts, braces, tension wires, fabric, tie wires, Class NS concrete, and all miscellaneous fittings and hardware to be included in the cost for Fencing, LF. Fencing shall be inclusive of the lengths of pull, end and corner post assemblies, but exclusive of gate widths.





(Pull, Corner, End And Approach Posts)

CONCRETE BASE FOR ANGULAR STEEL POST



FENCE POSITION AT LOCATIONS WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)

# DESIGN NOTE

This index details fencing that is constructed with farm fabric  $46\frac{1}{2}$ " (47" nominal) in height and with specific ground clearance and specific barbed wire spacings. For fencing of different height or installation details, the fence shall be fully detailed in the Contract plans.

**REVISION** 11/01/17

DESCRIPTION:

FDOT

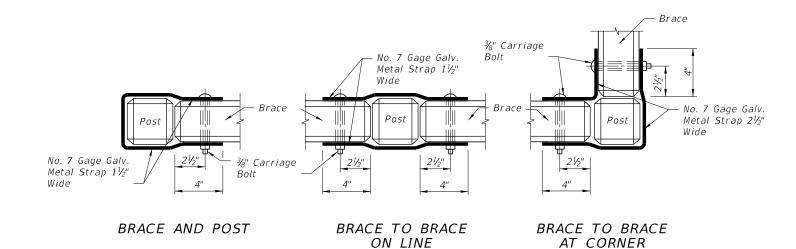
FY 2019-20 STANDARD PLANS

FENCE TYPE A

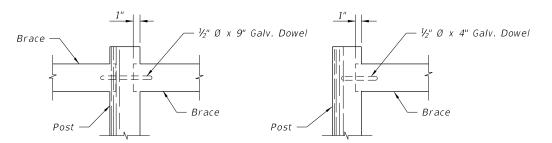
INDEX

SHEET

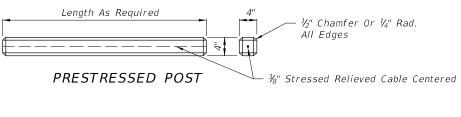
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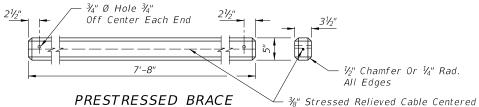


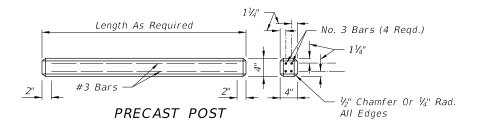
# FASTENER FOR CONCRETE POST AND BRACES

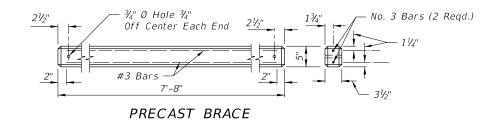


FASTENER FOR TIMBER POST AND BRACE

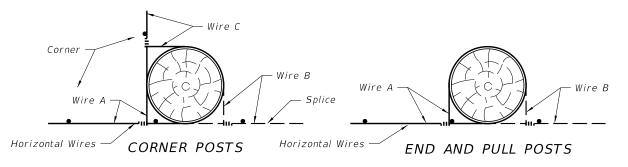








# ALTERNATE CONCRETE POSTS AND BRACES



Each horizontal wire to be wrapped around corner, end and pull posts and tied to same wire. See General Notes 5 and 17. Timber post illustrated. These methods also apply to steel and concrete post illustrations.

**SPLICES** 

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

FENCE TYPE A

INDEX

SHEET 3 of 3

550-001

## GENERAL NOTES

- 1. This fence to be used generally in urban areas.
- 2. For supplemental information refer to Specification 550.
- 3. Chain link fabric, post, truss rods, tension wires, tie wires, stretcher bars, gates and all miscellaneous fittings and hardware shall meet the requirements of AASHTO and ASTM signify current reference.
- 4. Fence Component Options:
  - A. Line post options:
  - (1) Galvanized steel pipe, Schedule 40-  $1\frac{1}{2}$ " nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table 2 (Grade A or B), ASTM F1083, and AASHTO M111.
  - (2) Aluminum coated steel pipe: ASTM A53, Table 2 (Grade A or B): Schedule 40- 11/2" nominal dia., 1.90" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
  - (3) Aluminum alloy pipe- 2" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
  - (4) Steel H-Beam- 11/8" x 15/8": Zinc Galv. 1.8 oz./ft.: AASHTO M111 and Detail.
  - (5) Aluminum alloy H-Beam- 17/8" X 15/8" Detail.
  - (6) Steel C- 1%"X 15%": Galv.: 1.8 oz/ft. zinc: AASHTO M111; OR , 0.9 oz./ft². zinc-5% aluminummischmetal: ASTM F1043 and Detail.
  - (7) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 2" OD, 11/2" NPS, 1.900" dec. equiv., 0.120" min. wall thick, and min. wt. 2.28 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15μg/in². min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.

  - B. Corner, end, and pull post options:

    (1) Galvanized steel pipe, Schedule 40- 2" nominal dia. zinc galvanized at the rate of 1.8 oz./ft².:

    ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.

    (2) Aluminum coated steel pipe: ASTM A53 steel, X 2 Tables: Schedule 40; 2" nominal dia.,

    2.375" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.

    - (3) Aluminum alloy pipe-  $2\frac{1}{2}$ " nominal dia.: ASTM B241 or B221, Alloy 6063,T6.
    - (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry  $2\frac{1}{2}$ " OD, 2" NPS, 2.375" dec. equiv., 0.130"min. wall thick. and min. wt. 3.117 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of  $15\mu g/in^2$ . min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.

#### C. Rail options:

- (1) Galvanized steel pipe, Schedule 40-  $1\frac{1}{4}$ " nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.
- (2) Aluminum coated steel pipe; ASTM A53 steel, X 2 Tables Schedule 40; 11/4" nominal dia., 1.660" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
- (3) Aluminum alloy pipe- 11/4" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
- (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 15/8" OD, 11/4" NPS, 1.660" dec. equiv., 0.111" min. wall thick. and min. wt. 1.836 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of  $15\mu g/in^2$ . min. and the polymer film topcoat shall have a thickness of 0.003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043
- D. Chain link fabric options (2" mesh with twisted and barbed selvage top and bottom for all options except as described in Note 10):
- (1) AASHTO M181 Type I Zinc Coated Steel, No. 9 gage (coated wire diameter), coated at the rate of 1.8 oz/ft². (M181 Class D 2.0 oz./ft². modified to 1.8 oz./ft².).
- (2) AASHTO M181 Type II -Aluminum Coated Steel, No. 9 gage (coated wire diameter), coated
- (3) AASHTO M181 Type IV- Polyvinyl Chloride (PVC) Coated Steel, No. 9 gage (coated core wire diameter), core wire-zinc coated steel. PVC coating: M181 Class A (either extruded or extruded and bonded) or Class B (bonded). See table right. Unless the plans call for M181 standard colors medium green, dark green or black the coating color shall be soft gray matching that of No. 36622 of Federal Standard 595a.

#### E. Tension wire options:

- (1) Steel wire No. 7 gage zinc galvanized at the rate of 1.2 oz./ft².: AASHTO M181.
- (2) Aluminum alloy wire with a diameter of 0.1875" or larger conforming to the requirements of ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
- (3) Aluminum coated steel wire No.7 gage coated at the rate of 0.040 oz./ft².: AASHTO M181.

# F. Tie wire and hog ring options:

- (1) Steel wire No.9 gage zinc galvanized at the rate of 1.2 oz./ft².
- (2) Aluminum alloy wire with a diameter of 0.1443" or larger conforming to the requirements of
- ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192. (3) Aluminum coated steel wire No. 7 gage coated at the rate of 0.040 oz./ft².

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(a) Only one fabric optional material will be permitted between corner and/or end post assemblies.

- (b) Only one line post optional material will be permitted between corner and/or end post assemblies.
  (c) Pull post assemblies shall be optional materials identical to either the line post optional material or the corner and end post assembly optional material; but, pull post assemblies shall be the same optional material between any set of corner and/or end post assemblies.
- 6. Concrete for bases shall be Class NS concrete as specified in Section 347 of the Standard Specifications or a packaged, dry material meeting the requirements of a concrete under ASTM C-387. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 7. Line post shall be 8'-6" long (Standard). Line post are to be set in concrete as described above or by the following methods:
  - (a) In accordance with special details and/or as specifically described in the contract plans and specifications.
  - (b) In accordance with ASTM F567 Subsections 5.4 through 5.10 as approved by the Engineer. Line post installed in accordance with Section 5.8 shall be 9'-6" long.
  - (c) Post mounted on concrete structure or solid rock shall be mounted in accordance with the base plate detail "Fence Mounting On Concrete Endwalls And Retaining Wall", Sheet 3; or, by embedment in accordance with ASTM F567 Subsection 5.5.

End, pull and corner post assemblies shall be in concrete as detailed above for all soil conditions other than solid rock. Post within assemblies that are located on concrete structures or solid rock shall be set by base plate or by embedment as prescribed under (b) above for line post.

Line and assembly posts for 6' fence which must be lengthened due to a variation in the normal ground clearance, shall be set an additional 3" in depth for each 1' of of additional ground clearance.

- 8. Pull post shall be used at breaks in vertical grades of 15° or more, or at approximately 350' centers except that this maximum interval may be reduced by the Engineer on curves where the curve is greater than 3°.
- 9. Corner post are to be installed at all horizontal breaks in fence at 15° or more and as required at vertical breaks over 15° as determined by the Engineer.
- 10. When fence has an installed top of fabric height less than 6' knuckled top and bottom selvages shall be used unless the plans specifically identify locations for twisted selvage fabrics.
- 11. Unless sliding gates or special gates are called for in the plans, all gates shall be chain link swing gates meeting the material requirements described and as approved by the Engineer. Payment shall include the gates, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 12. For construction purposes corner post assemblies shall consist of one corner post, two braces, two truss rods, and all necessary fittings and hardware as detailed. End post assemblies shall consist of one end post, one brace, one truss rod and all necessary fittings and hardware as detailed.
- 13. In areas where there are physical constraints outside the right-of-way which restricts the fence construction, the fabric may be installed on the inside of the posts..

TYPE IV VINYL COATED FABRIC									
	AASHTO M181 Table 4 Redefined As Follows								
				PVC Thickness Range					
Of Me	Specified Diameter Of Metallic Coated Core Wire  Minimum We Of Zinc Coa			M181 Class A (Extruded Or Extruded And Bonded Coating)		M181 Class B (Bonded Coating)			
in.	mm	gage	oz./ft².	g/m²	in.	mm	in.	mm	
0.148	3.77	9	0.30	92	0.015 to 0.025	0.38 to 0.64	0.006 to 0.010	0.15 to 0.25	

#### DESIGN NOTE

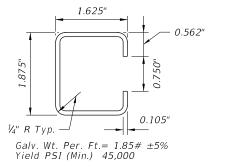
This index details fencing that is constructed with chain link fabric 6' (nominal) in height and with specific ground clearance.

For fencing of different height or installation details, the fence shall be fully detailed in the Contract plans.

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LAST REVISION 11/01/17

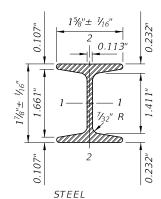




## STANDARD WALL

# THINWALL

#### OPTIONAL "C" LINE POST



Area (Sq. In.)	
Weight (Lb./Ft.)	
Surface Area (SF/Ft.)	
Tensile Strength (psi Min.)	
Yielding Point (psi Min.)	

2.72 ± 5% (Galv.) 80,000

•	· ·
<i>Axes</i>	Axes
1-1 2-2	1-1 2-2
428 0 101	0.428 0.101

ALUMINUM

 $0.91 \pm 5\%$ 0.776

30,000

#### Moment Of Inertia Section Modulus 0.456 0.124 0.456 0.124 Rad. Of Gyration 0.779 0.373 0.779 0.373

OPTIONAL 17/8" x 15/8" H-BEAM LINE POST

# Steel % " R (ASTM A36) Galvanized or Aluminum ¾" ₽ Alloy 6061-T6 %₁6" Dia. Hole For √2" Galvanized Anchors, Nuts And Washers (4 Regd.) Dimensions Same As Adjacent Side

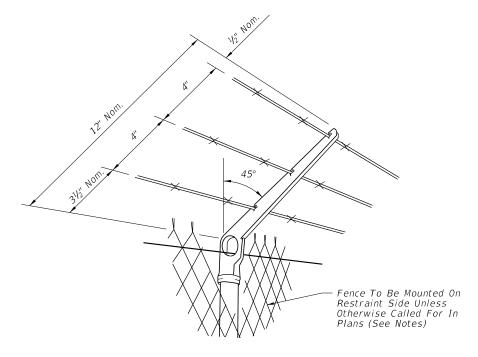
TOP VIEW FOUR ANCHOR PLATE OPTION

DESCRIPTION:

# R/W Line 6" Or As Indicated In Plans Wire Fabric -Private Property

# FENCE POSITION AT LOCATIONS WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)



# **NOTES**

Attachments to be used only when called for in the plans.
Attachments to extend in direction of restraint. Unless otherwise called for in plans, direction of restraint will be as follows:

(a.) Outward on limited access right of way line.

(b.) Outward on controlled access right of way line.

(c.) Outward from utilities and hazardous facilities located within highway right of way.

- within highway right of way.
- (d.) Outward from lateral ditches, outfalls, retention basins, canals, borrow areas and similar support facilities.
- (e.) Inward on pedestrian ways.

The cap-arm shall be designed to provide a drive fit over the top of posts and to exclude moisture in posts with tubular sections.

#### BARB WIRE ATTACHMENT

#### BASE PLATE AND ANCHOR NOTES:

- 1. Base plate identical for line, pull, end and corner posts and shall be considered an integral part of the respective posts for basis of payment.
- 2. Post to be plumbed by grout shim under base plate.
- 3. Anchors (Galvanized Steel):

12" Cast In Place, 10½" Embedment: Headed Bolts, U-Bolts or Cluster Plates. 8" Adhesive Anchors, 6" Min. Embedment.*

*Adhesive anchors shall be headless anchor bolts set in drilled holes with an Adhesive Material System in accordance with Specifications 416 and 937; drilled holes shall be  $\frac{1}{8}$ " larger in diameter than the anchor bolt.

Expansion Bolts Not Permitted.

# TOP VIEW TWO ANCHOR PLATE OPTION

7/8" Dia. Hole For ¾"Anchors,

Nuts And Washers (2 Regd.)

11/4"

# FENCE MOUNTING ON CONCRETE ENDWALL AND RETAINING WALLS

└─ Steel ½" ॡ (ASTM A36) Galvanized or

Aluminum 1/2" P2 Alloy 6061-T6

**REVISION** 11/01/17

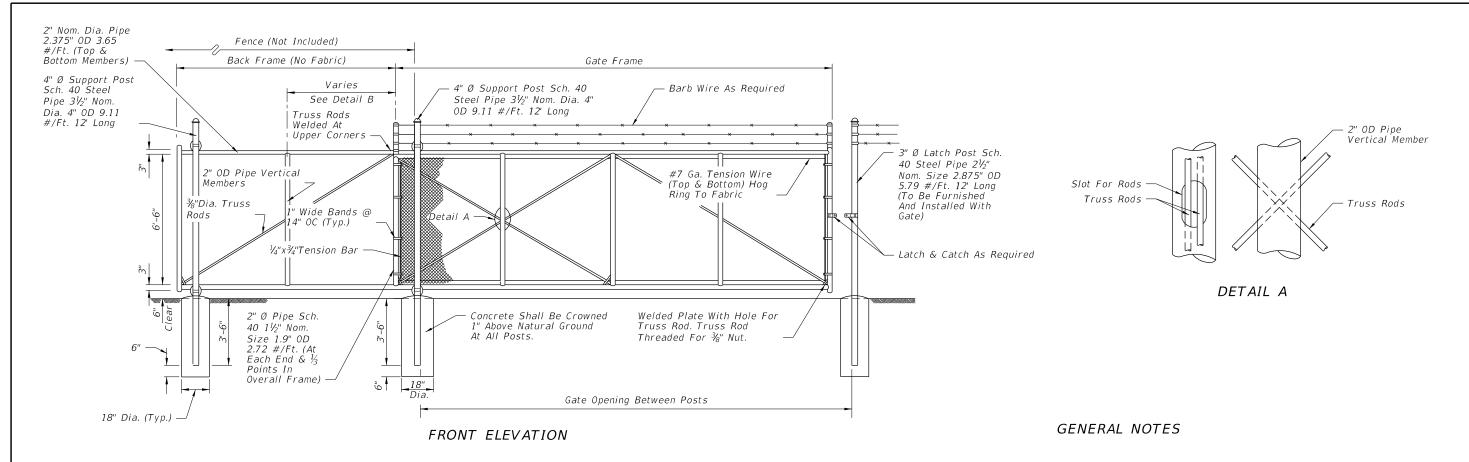


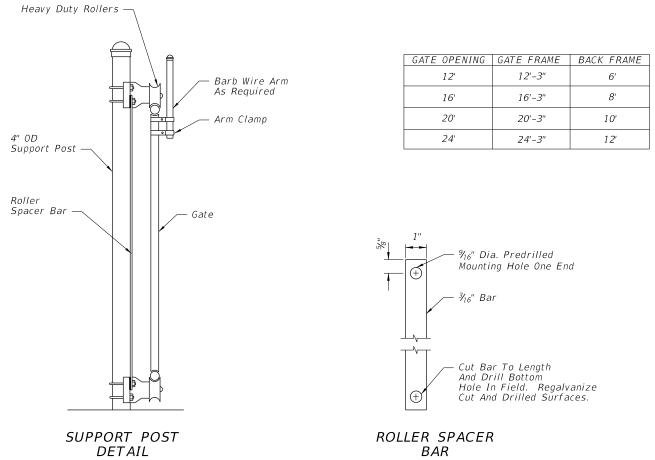
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INDEX FENCE TYPE B *550-002* 

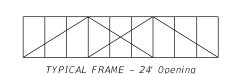


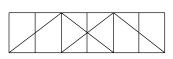


1. Extruded, rolled or formed components that provide equal strength and stability may be used in lieu of the pipe components shown; and, internal rollers may be used in lieu of the external roller units shown.

Gate components shall meet or exceed the protective coatings specified on Index 550-002.

- 2. Steel gate frame shall be fabricated prior to galvanizing, except that truss rods may be fabricated following frame galvanizing provided surfaces damaged during welding are galvanized in accordance with Section 24 of AASHTO M36; or, fabricated from pipe components with protective coating meeting the requirements of Index 550-002 that are tolerant of welding (low burn back), and a protective coating applied to the weld and damaged pipe surfaces that is equivalent to the protective coating of the fabricated pipe
- 3. All fabric shall be knuckled top and bottom selvages.
- 4. Concrete for bases shall be either Class NS concrete in accordance with Specification 347 or a packaged, dry material meeting the requirements of a concrete under ASTM C-387. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 5. Cost of all gate components shall be included in the contract unit price for Sliding Fence Gate (Cantilever), EA.





TYPICAL FRAME - 12', 16' & 20' Opening

DETAIL B

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DESCRIPTION:

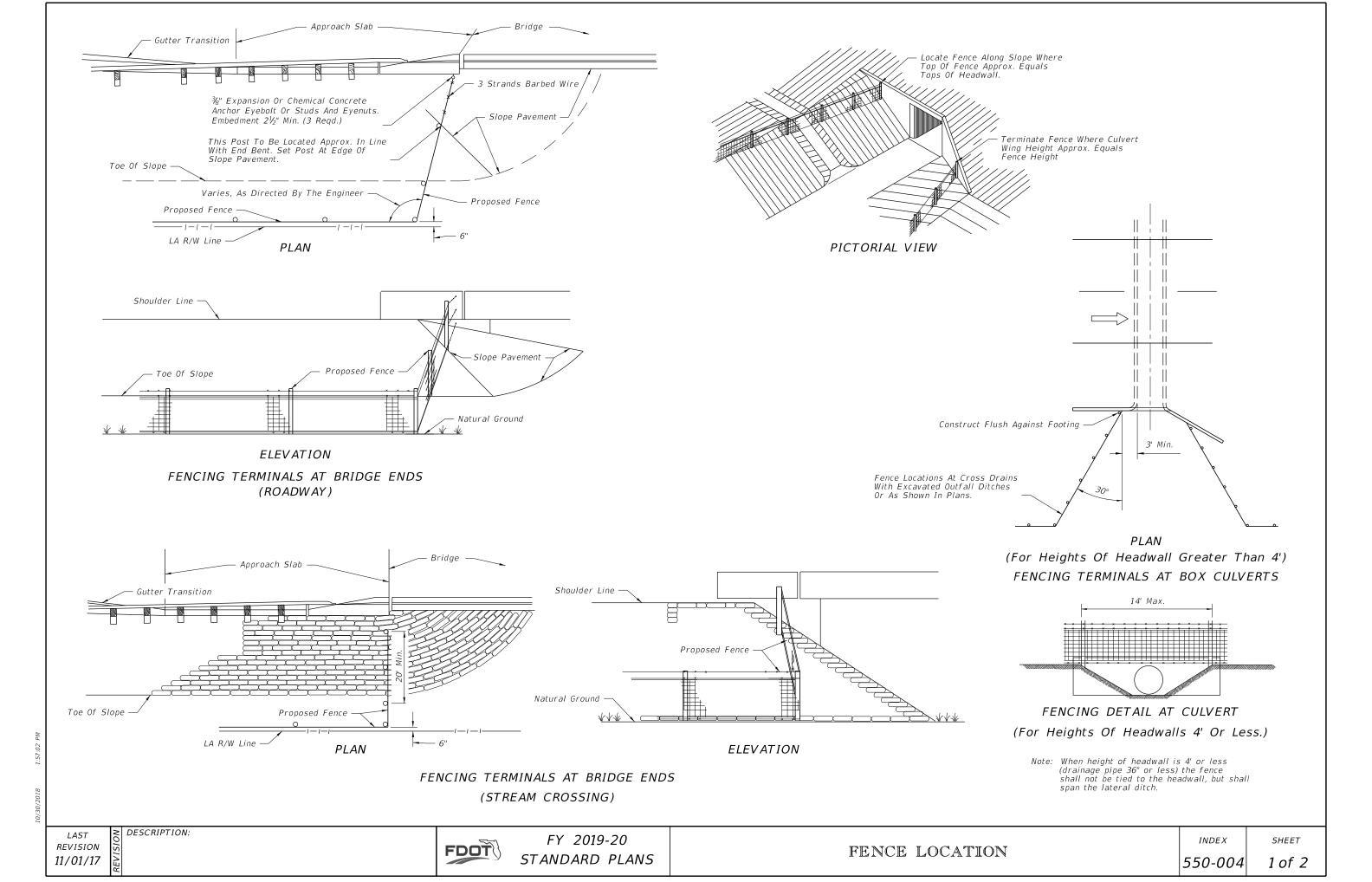
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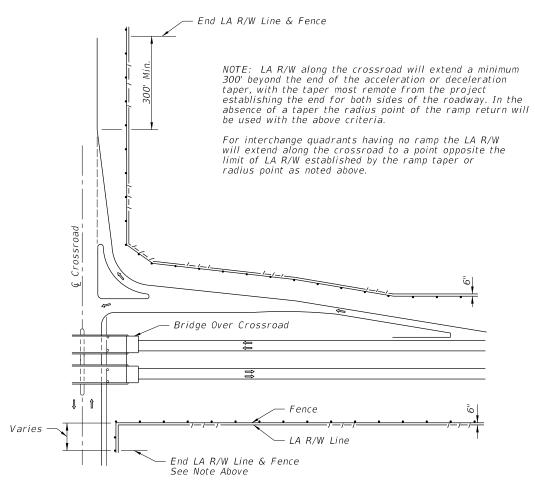
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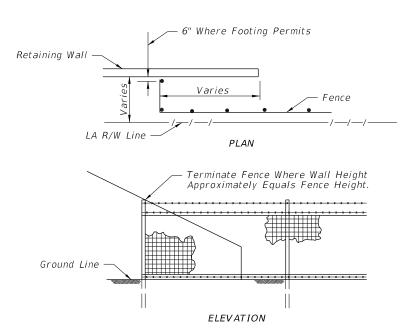
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APPLIES TO BRIDGE OVER CROSSROAD AND CROSSROAD OVER FREEWAY (BRIDGE OVER CROSSROAD SHOWN)

# FENCING TERMINALS AT RURAL INTERCHANGES



FENCING TERMINALS AT RETAINING WALLS

DESCRIPTION: LAST **REVISION** 11/01/17



— @ Cross Street

- See Note B

See Note A

End Fence & LA R/W Line

Radius Point

— Sidewalk

INSET A

LA R/W Line -

Fence Type "B"

Fence Type "B"

dimension, if practical.

50' Min. Overlap

Local Street —

Note A - The indicated distance shall be sufficient to provide satisfactory

Note B - The indicated distance shall be identical to the above noted

FENCING TERMINALS AT URBAN INTERCHANGES

sight distance for the traffic from the ramp.

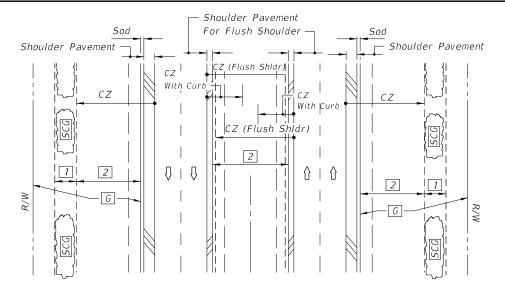
- See Inset A

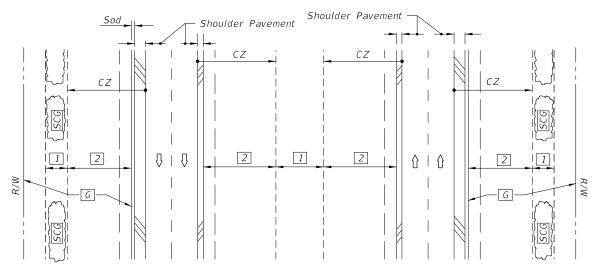
LA R/W Line

Fence Type "B"

50' Min. Overlap -

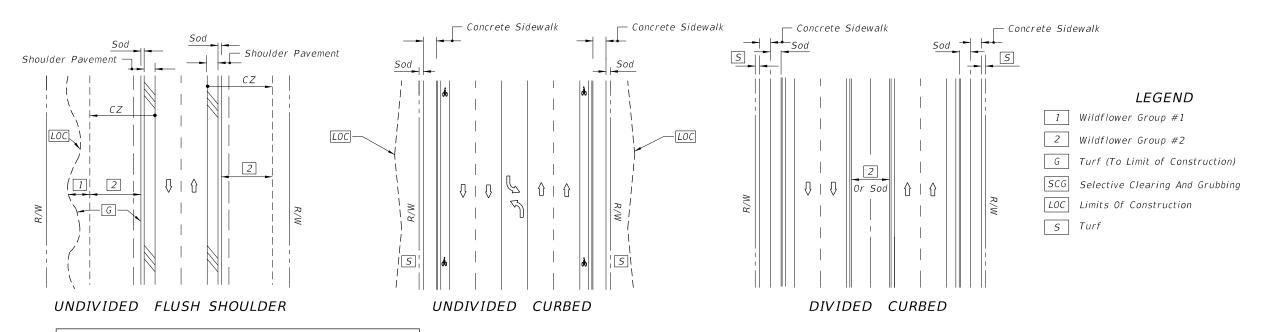
SHEET





# DIVIDED NARROW MEDIAN WITH OR WITHOUT CURBED MEDIAN

# DIVIDED WIDE MEDIAN WITH OR WITHOUT CURBED MEDIAN



WILDFLOWER SEEDING RATES				
Common Name (Botanical Name)	lbs/ac			
#1 Group				
Black-Eyed Susan (Rudbeckia hirta)	2			
Lance-Leaf Tickseed (Coreopsis lanceolata)	10			
Goldenmane Tickseed (Coreopsis basalis)	10			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Softhair Coneflower (Rudbeckia mollis)	2			
Crimson Clover (Trifolium incarnatum)	15			
#2 Group				
Annual Phlox (Phlox drummondii)	10			
Moss Verbena (Verbena tenuisecta)	6			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Crimson Clover (Trifolium incarnatum)	15			
Note: Wildflower seeding rates are for restoring impacted wildflower areas.				

# GENERAL NOTES

- 1. All turf establishment shall be performed meeting the requirements of Specification 570.
- 2. Activities such as clearing, grading, and excavating that will disturb one or more acres of land require coverage under the Generic Permit for Stormwater Discharge from Large and Small Construction Activities from the Florida Department of Environmental Protection, and implementation of appropriate pollution prevention measures to minimize erosion and sedimentation and properly manage stormwater.
- 3. Confirm compatibility of wildflower with Seeding Zones.

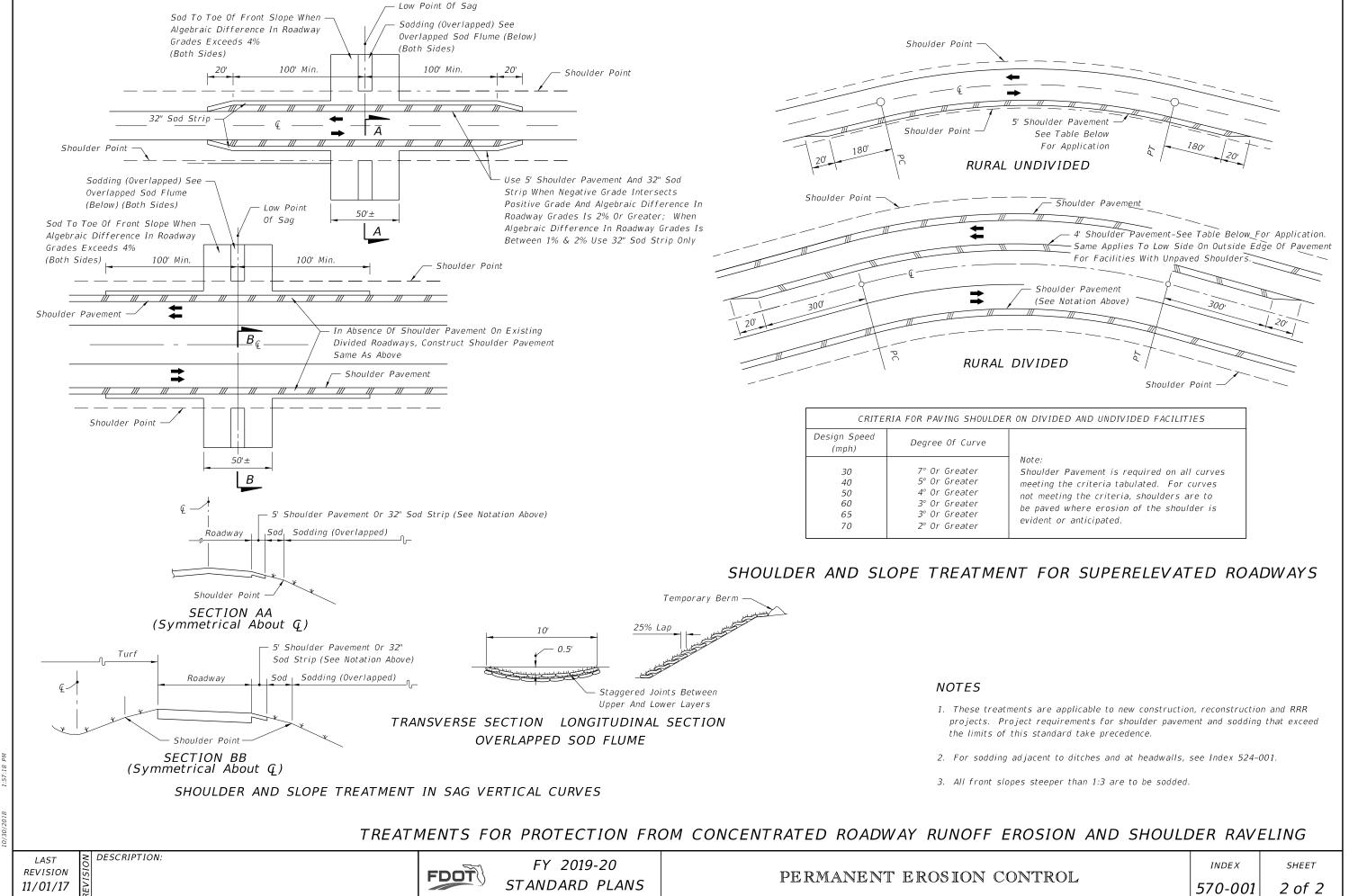


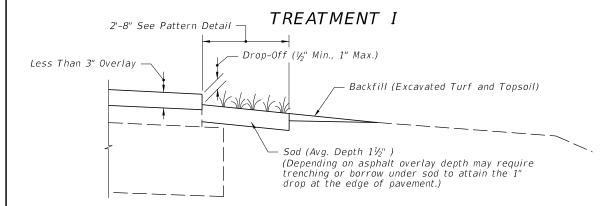


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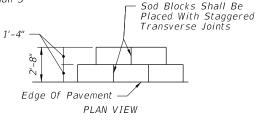




# COMPLETED SHOULDER

CRITERIA FOR USING TREATMENT I

- is resurfacing, widening and resurfacing or construction of shoulder pavement
- is rural or is urban without curb and gutter
   resurfacing build-up is less than 3"



## PATTERN DETAIL

#### GENERAL NOTES

#### 1. Treatment I:

If trenching under sod is necessary to achieve the required Drop-Off, excavated topsoil is to be used for filling voids and low areas at the edge of pavement or for flushing along the edge of sod. Excess material to be uniformly distributed over the shoulder.

#### 2. Treatment II:

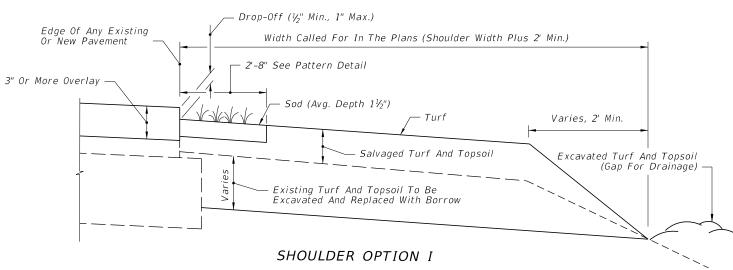
- A. Borrow must meet the requirements for a "Select" material in accordance with Index 120-001 and Specification 120.
- B. Borrow may be used in lieu of excavated turf and topsoil when economically feasible. There will be no additional payment for substituting borrow for excavated turf and topsoil.
- 3. Special attention is to be directed at achieving the required Drop-Off at the edge of pavement, within the dimension range shown.
- 4. Activities such as clearing, grading, and excavating that will disturb one or more acres of land require coverage under the Generic Permit for Stormwater Discharge from Large and Small Construction Activities from the Florida Department of Environmental Protection, and implementation of appropriate pollution prevention measures to minimize erosion and sedimentation and properly manage stormwater.

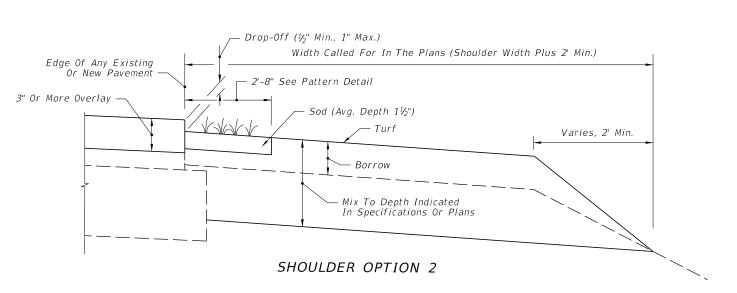
#### 5. Turf Establishment:

DESCRIPTION:

- A. Wildflowers destroyed by shoulder sodding and turf operations are to be reestablished under the seeding rates prescribed for permanent wildflower #2 Group shown by table on Index 570-001.
- B. Establish turf in accordance with Specification 570.

# TREATMENT II





#### CRITERIA FOR USING TREATMENT II

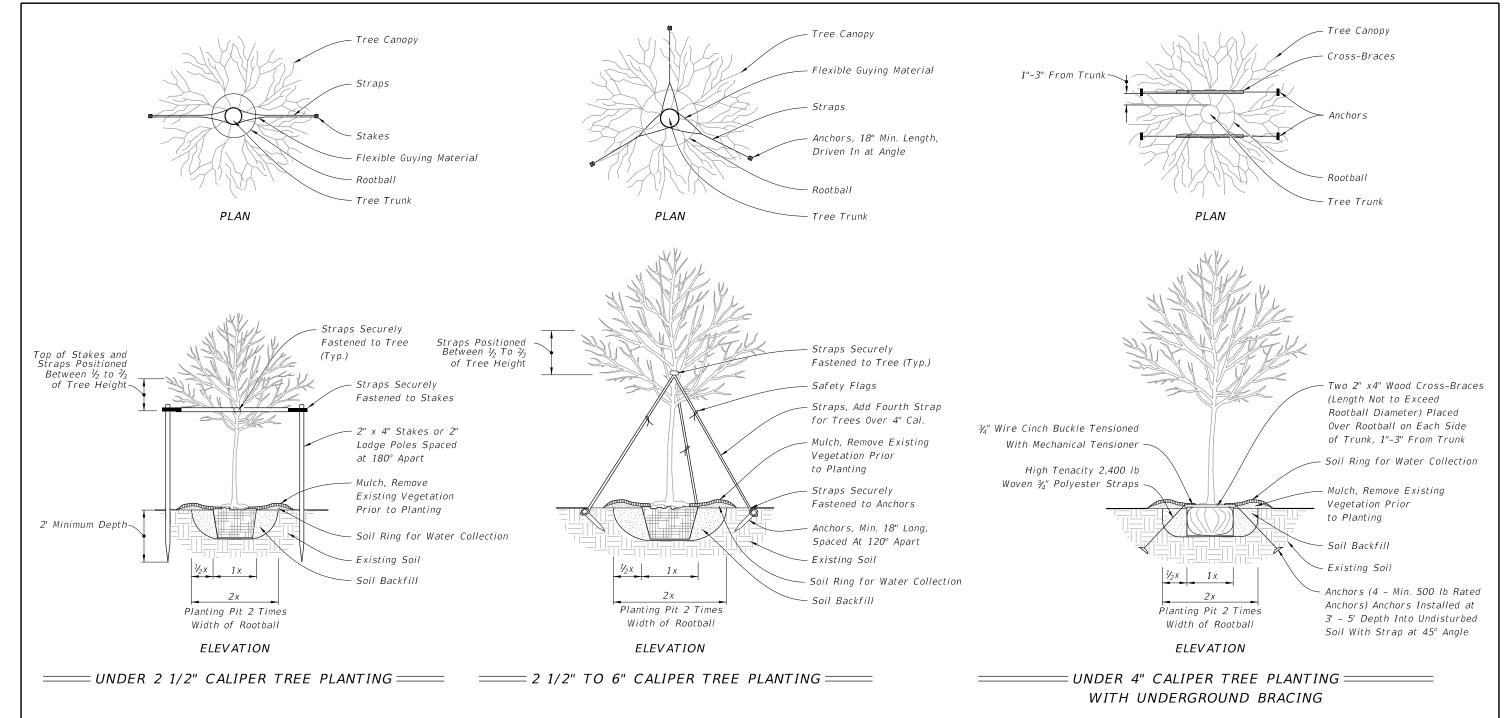
#### Project

- is resurfacing or construction of shoulder pavement
- is rural or is urban without curb and gutter
- resurfacing build-up is 3" or more

A SIMILAR TREATMENT MAY BE USED FOR PROJECTS THAT REQUIRE SHOULDER WIDENING. DETAILS ARE TO BE SHOWN IN THE PLANS.

**REVISION** 11/01/18





#### **GENERAL NOTES:**

- 1. All dimensions 6" and less are exaggerated for illustrative purposes only. All dimensions provided for wood materials are nominal.
- 2. Remove plant containers prior to planting. If plants are not container grown, remove a minimum of the top 1/3 of burlap, fabric, or wire mesh.
- 3. Allow no more than 1" of soil to cover the uppermost root on all trees. Set the top of root ball 1"-2" above finish grade after settling and set plumb to the horizon.
- 4. Backfill with loosened existing soil or as shown in the plans. Remove rocks, sticks, or other deleterious material greater than 1" in any direction prior to backfilling. Water and tamp to remove air pockets. If existing soils contain excessive sand, clay, or other material not conducive to proper plant growth, contact Engineer prior to planting.

- 5. Except when a permanent, subsurface or drip irrigation system is provided, construct soil rings at the outer edge of the planting pit, with a height of 3" and gently sloping sides. Do not pile soil on top of rootball.
- 6. Construct a 3" deep layer of mulch placed 2" off the edge of the trunk flare, around the base of shrub, or solidly around ground cover. Never pile mulch against the tree trunk.
- 7. Install guying with minimum 1" wide nylon or polypropylene straps. Check straps monthly and adjust as required to eliminate girdling of tree. Locate all wood stakes or anchors beyond the edge of soil ring in undisturbed soil and located below finished grade, unless otherwise specified. Alternate tree bracing and guying systems specified or approved by the Engineer may be used in lieu of the tree bracing and guying methods detailed on the Index.
- 8. Relocated Trees and Palms: Brace relocated trees and palms in accordance with the Contract Documents. Remove bracing at the conclusion of the contract or as directed by the Engineer. Bracing or straps must not damage or become embedded in the tree bark.

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DESCRIPTION:

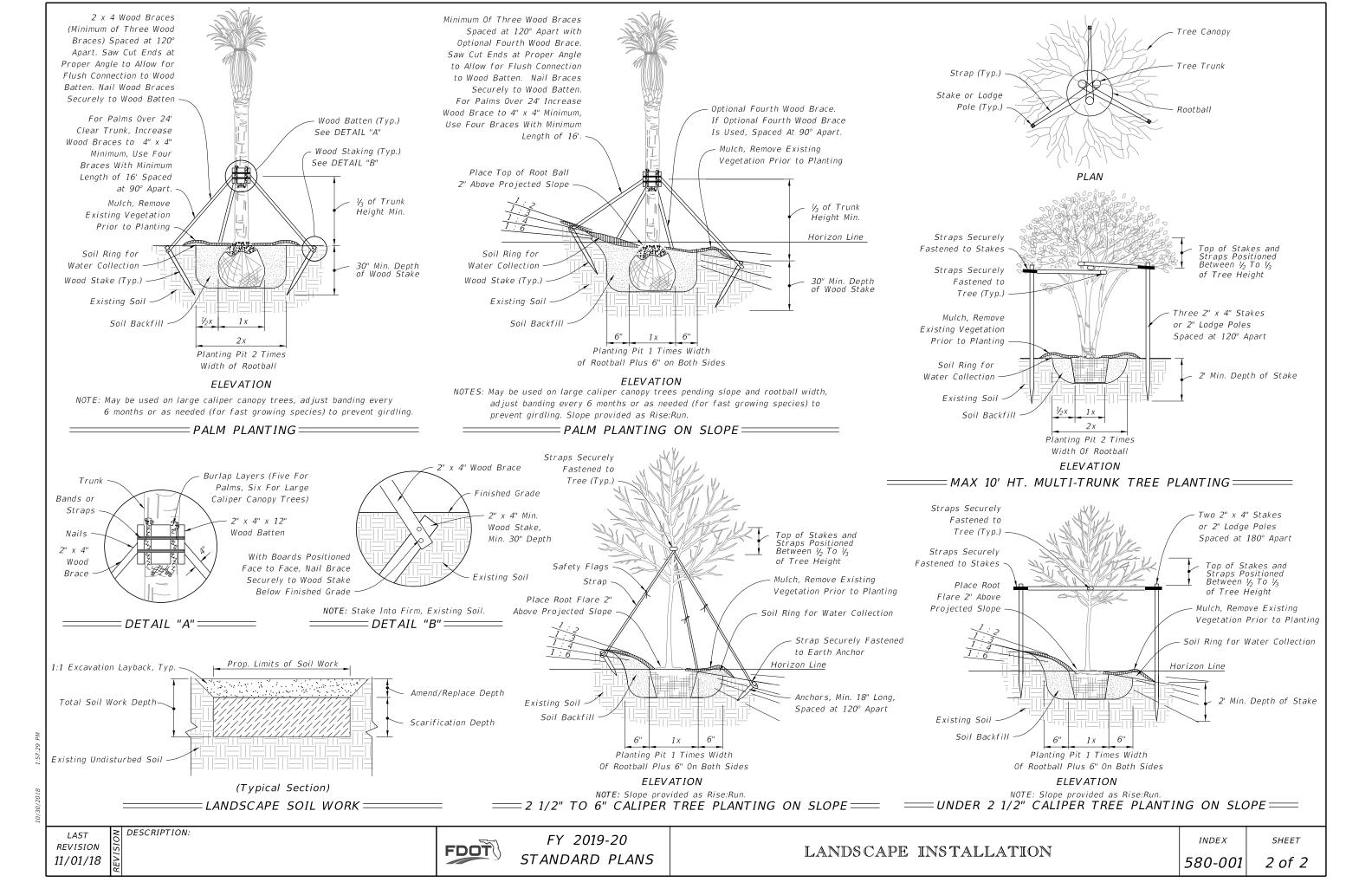
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LANDSCAPE INSTALLATION

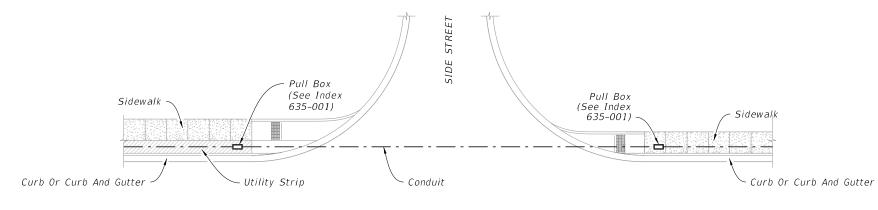
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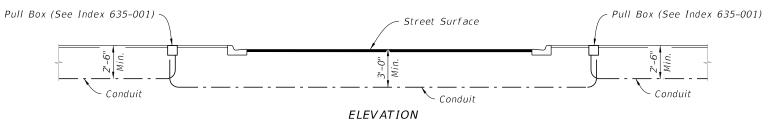


# GENERAL NOTES:

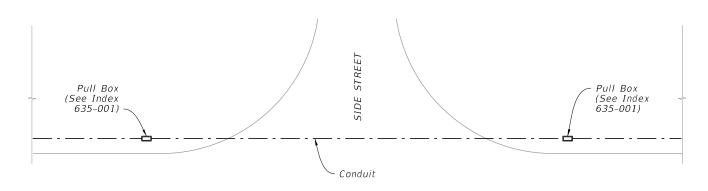
- 1. Install conduit in accordance with Specification 630.
- 7. When installing conduit under sidewalk by open trench, replace the entire sidewalk slab.
- 3. Trench not to be open more than 250' at a time when construction area is subject to vehicular or pedestrian traffic.
- 4. Sawcut asphalt at the edges of the trench to leave neat lines.



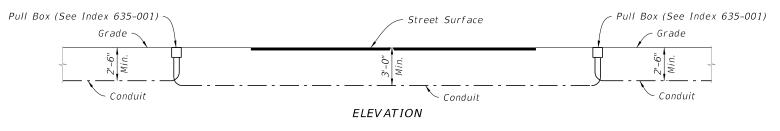
PLAN



CURB AND GUTTER



PLAN

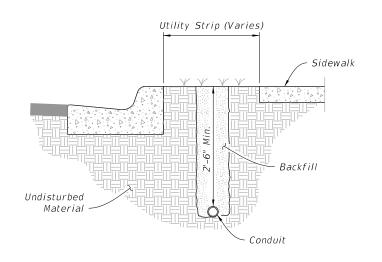


=FLUSH SHOULDER=

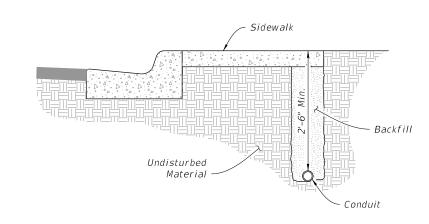
REVISION 11/01/18

DESCRIPTION:

FDOT



# = PLACEMENT WITHIN THE UTILITY STRIP ====



3'-0" Min. or as Directed by the Engineer

Backfill

Conduit(s)

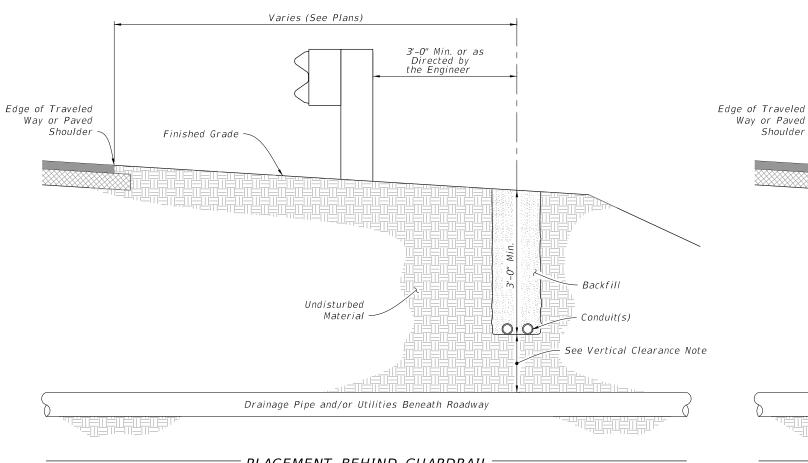
# = PLACEMENT UNDER SIDEWALK ===

Varies (See Plans)

Finished Grade

Undisturbed

Material



See Vertical Clearance Note Drainage Pipe and/or Utilities Beneath Roadway

# PLACEMENT BEHIND GUARDRAIL

:PLACEMENT IN FRONT OF GUARDRAIL=

# VERTICAL CLEARANCE NOTE:

Maintain 1'-0" minimum vertical clearance when crossing over pipe and or utilities. If minimum vertical clearance cannot be maintained, conduit is to be routed under pipe maintaining 1'-0" minimum vertical clearance.

. Way or Paved

Shoulder

**REVISION** 11/01/18

DESCRIPTION:

FDOT

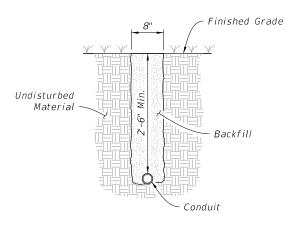
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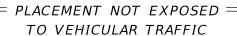
CONDUIT INSTALLATION DETAILS

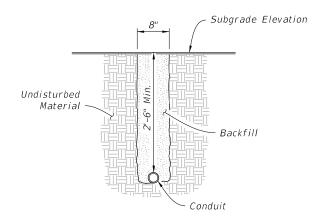
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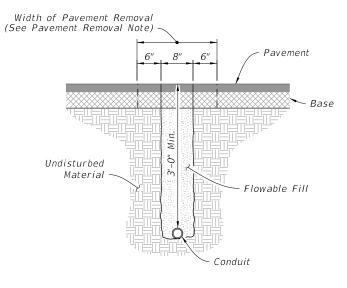




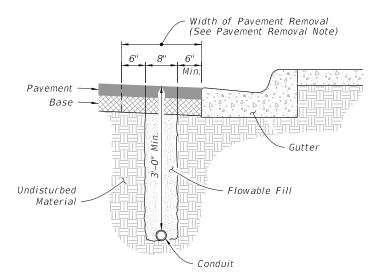
= PLACEMENT UNDER NEW ROADWAY ==PRIOR TO INSTALLATION OF BASE AND PAVEMENT

# NOTES:

- 1. Pavement Removal: The removal and replacement of the additional pavement width (i.e., 6" Width either side of trench) will not be required when the trench can be constructed without disturbing the asphalt surface on either side.
- 2. Placement Under Existing Pavement: Place conduit prior to installation of base and pavement, unless otherwise shown in the Plans or approved by the Engineer.

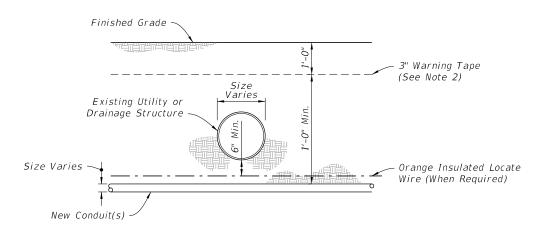


=PLACEMENT UNDER EXISTING PAVEMENT==NOT ADJACENT TO GUTTER

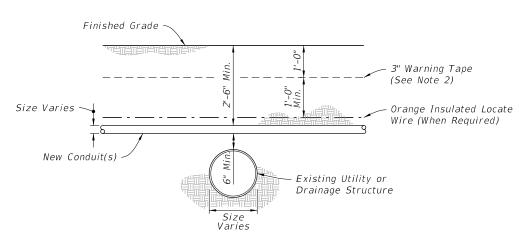


PLACEMENT UNDER EXISTING PAVEMENT ADJACENT TO GUTTER

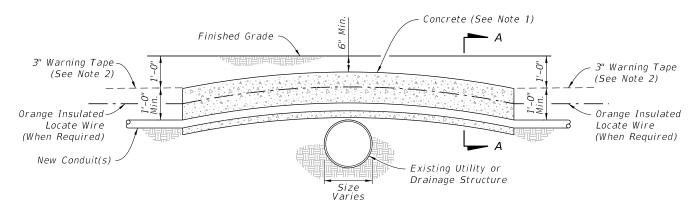
**REVISION** 11/01/18



#### BELOW EXISTING

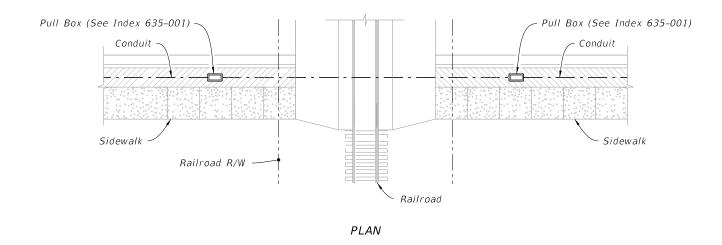


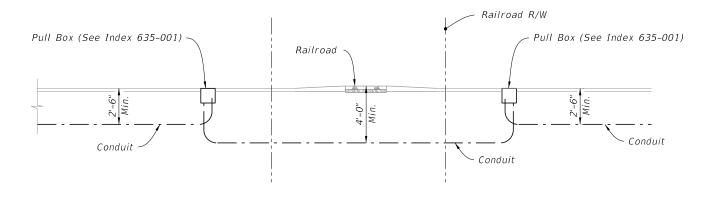
ABOVE EXISTING - DEPTH 2'-6" OR GREATER



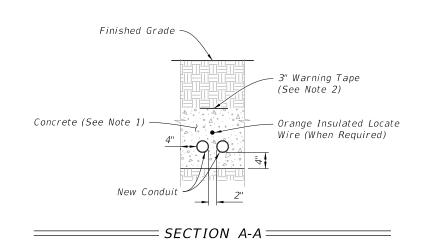
ABOVE EXISTING - DEPTH 2'-6" OR LESS

PLACEMENT ACROSS EXISTING DRAINAGE PIPES OR UTILITIES





# PLACEMENT UNDER RAILROAD =



# NOTES:

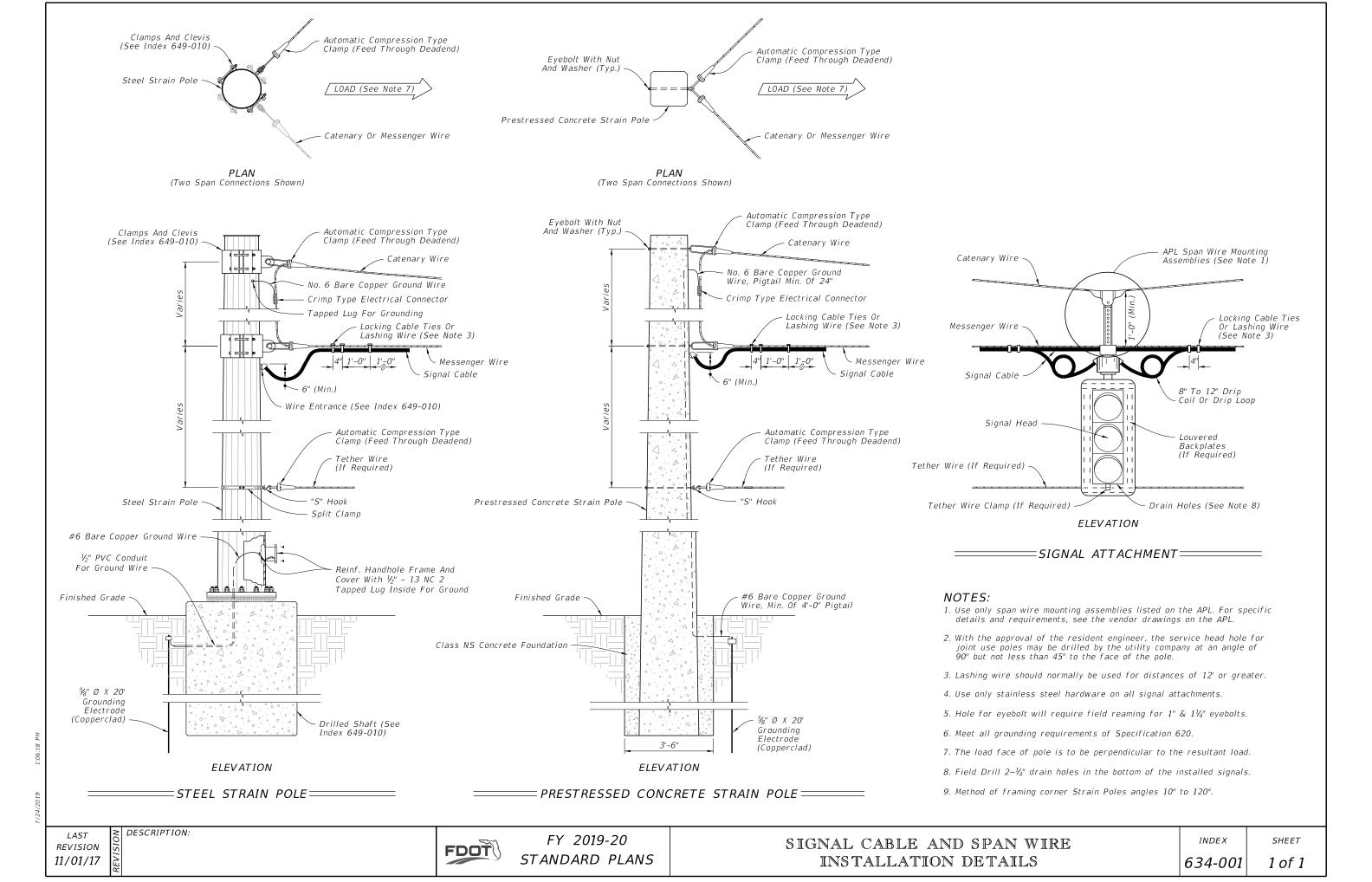
- 1. Where conduits are to be installed over existing underground structures (e.g., drainage pipes or utility lines) which are less than 2'-6" deep, encase the conduit in Class NS concrete for the entire length of conduit that is installed at a depth of less than 2'-6".
- 2. Place 3" Warning Tape when new conduit is installed at a depth of 1'-6" or greater, and the new conduit is not encased in

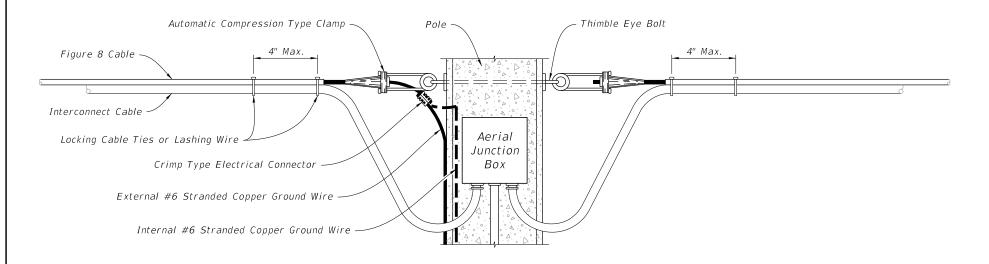
LAST **REVISION** 11/01/18

DESCRIPTION:

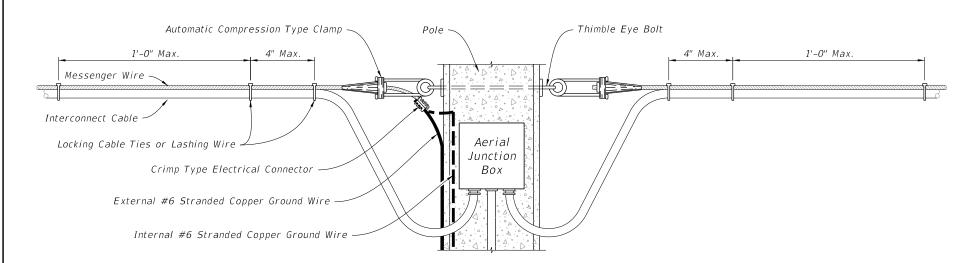
FDOT

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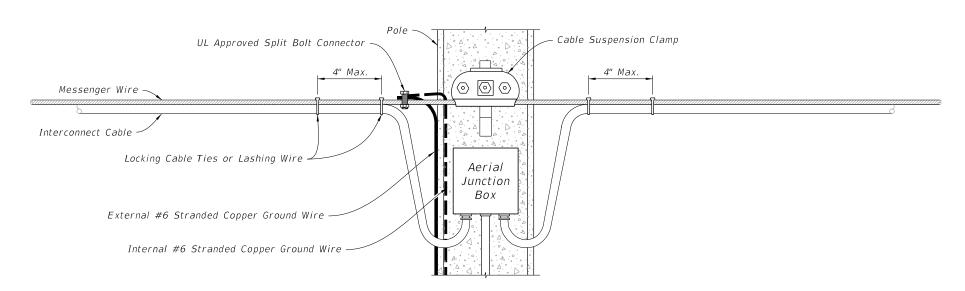




#### CABLE DROP AND TERMINATION WITH FIGURE 8 CABLE =



#### CABLE DROP AND TERMINATION WITH MESSENGER WIRE AND COMPRESSION CLAMP=



CABLE DROP AND TERMINATION WITH MESSENGER WIRE AND SUSPENSION CLAMP =

LAST **REVISION** 11/01/18

DESCRIPTION:

**FDOT** 

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# AERIAL INTERCONNECT

NOTES:

extending down the pole.

1. Meet all grounding requirements of Specification 620.

2. If accessible, ground the messenger wire of the interconnect cables to the copper ground wire of the pole or to the external wire

conduit extending up 8' from the finish grade to protect the ground

4. Use either locking cable ties or lashing wire, placed no further than 12" apart. Except at the point of cable drop or terminations, place

one (1) at the point where the cables separate from the messenger wire and place another at a maximum distance of 4" from that tie.

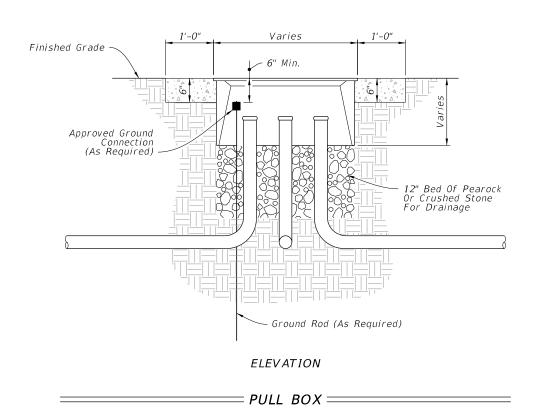
5. When installing Figure 8 interconnect cable, only use locking cable ties. 6. Lashing wire should normally be used for distances of 12' or greater.

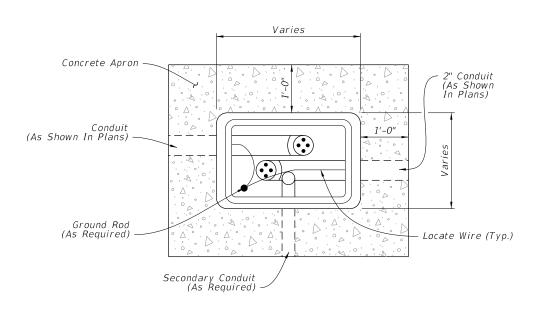
3. When utilizing the external ground wire, install a piece of 1/2"

wire connecting the messenger wire to the ground rod.

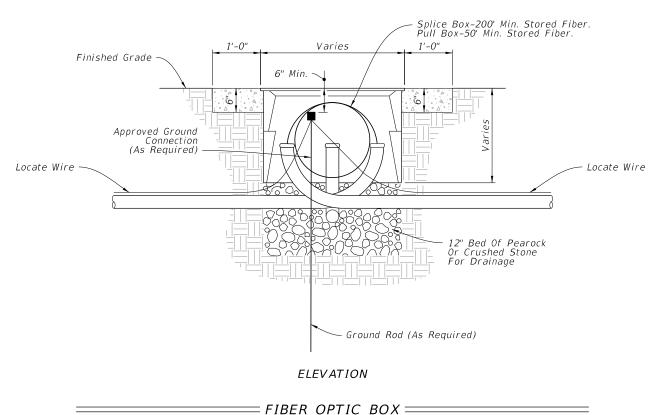
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#### PLAN





#### PLAN



#### NOTES:

- 1. Provide fiber optic splice boxes with cable hanger racks designed to support cables and splice enclosures.
- 2. Install a 1'-0" wide (Min.) concrete apron around all boxes using Class NS concrete. Slope the apron away from the box.
- 3. Where multiple pull boxes are placed side by side, maintain at least 8" between the pull boxes.
- 4. Rectangular boxes shown, others similar.

LAST **REVISION** 11/01/18

DESCRIPTION:

FDOT

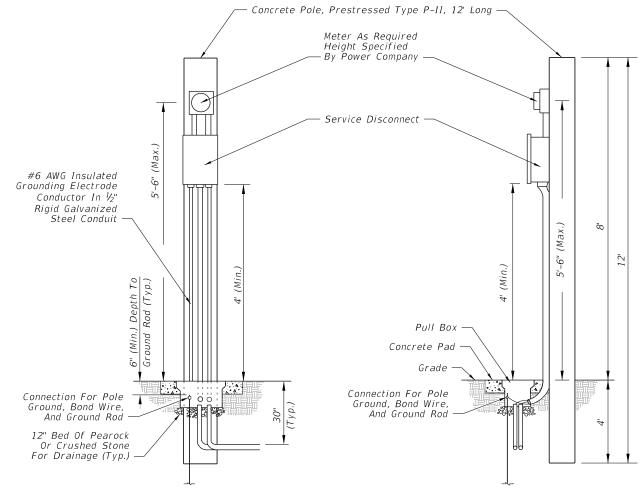
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# Clevis With Insulators Conductor Weatherhead Height As Required By Power Company Meter As Required Height Specified By Power Company Service Disconnect #6 AWG Insulated Grounding Electrode (Max.) Conductor In 1/2" Rigid Galvanized Steel Conduit 2,-6" Pull Box Concrete Pad Grade 12" Bed Of Pearock Or Crushed Stone For Drainage (Typ.) U.L. Approved Ground Rod, ⅓" Dia. 40' Long Copper Clad (All Service Points)

Concrete Pole Prestressed Type P-II, 36' Long -

#### **GENERAL NOTES:**

- 1. It shall be the contractors responsibility to provide a complete service assembly as per the plans and service specifications.
- 2. The service installation shall meet the requirements of the national electric code and applicable local codes.
- 3. Shop drawings are not required for service equipment, unless noted in the plans.
- 4. A Pull Box is required at each service point, see Index 635-001.



DETAIL B UNDERGROUND FEED

DESCRIPTION: **REVISION** 

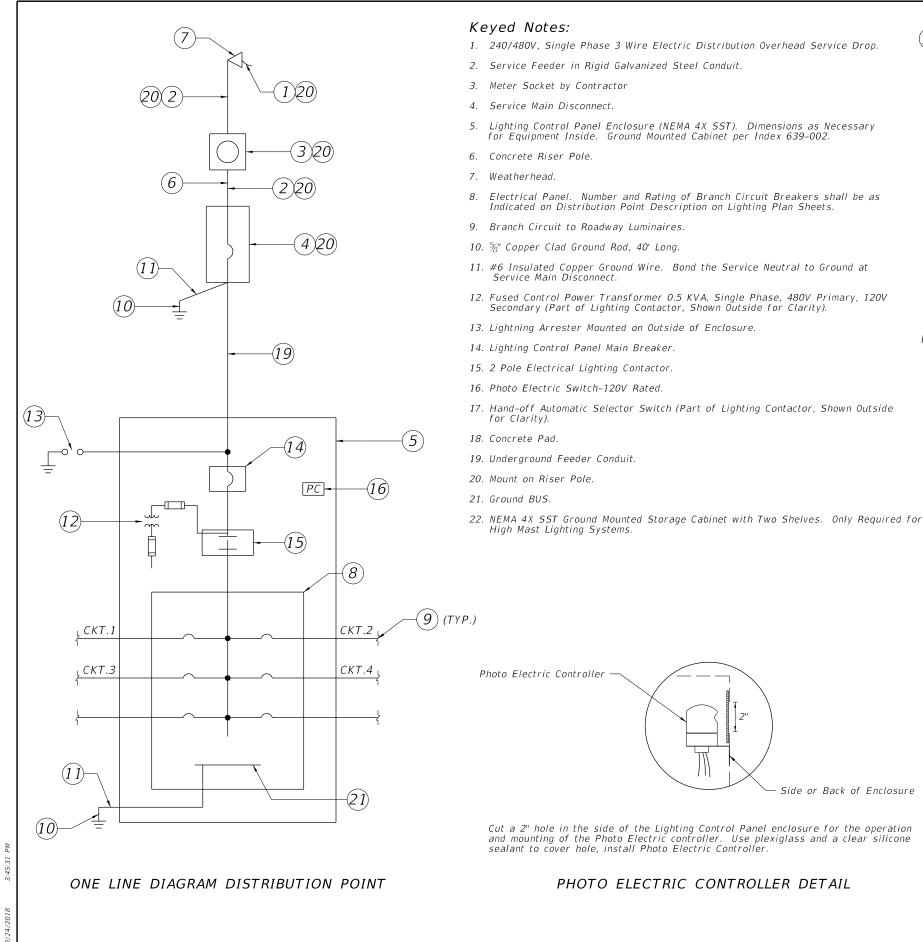
DETAIL A

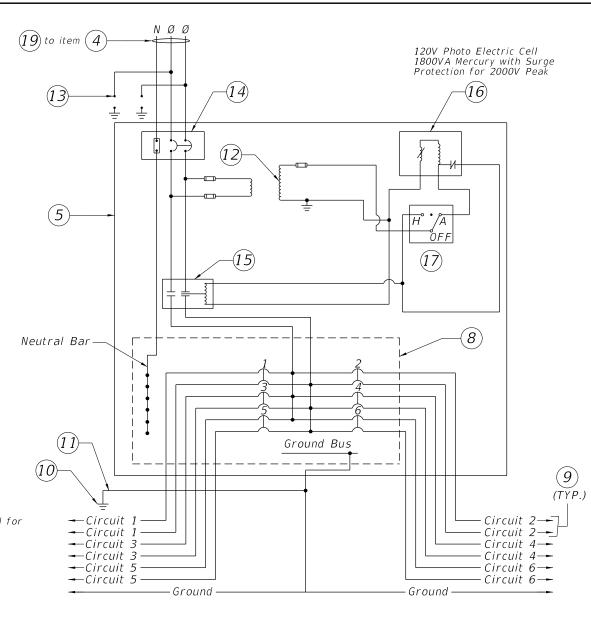
AERIAL FEED

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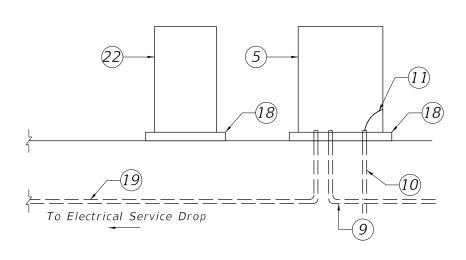
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#### TYPICAL DISTRIBUTION POINT SCHEMATIC DETAIL



RISER DIAGRAM - TYPICAL DISTRIBUTION POINT

**REVISION** 11/01/17

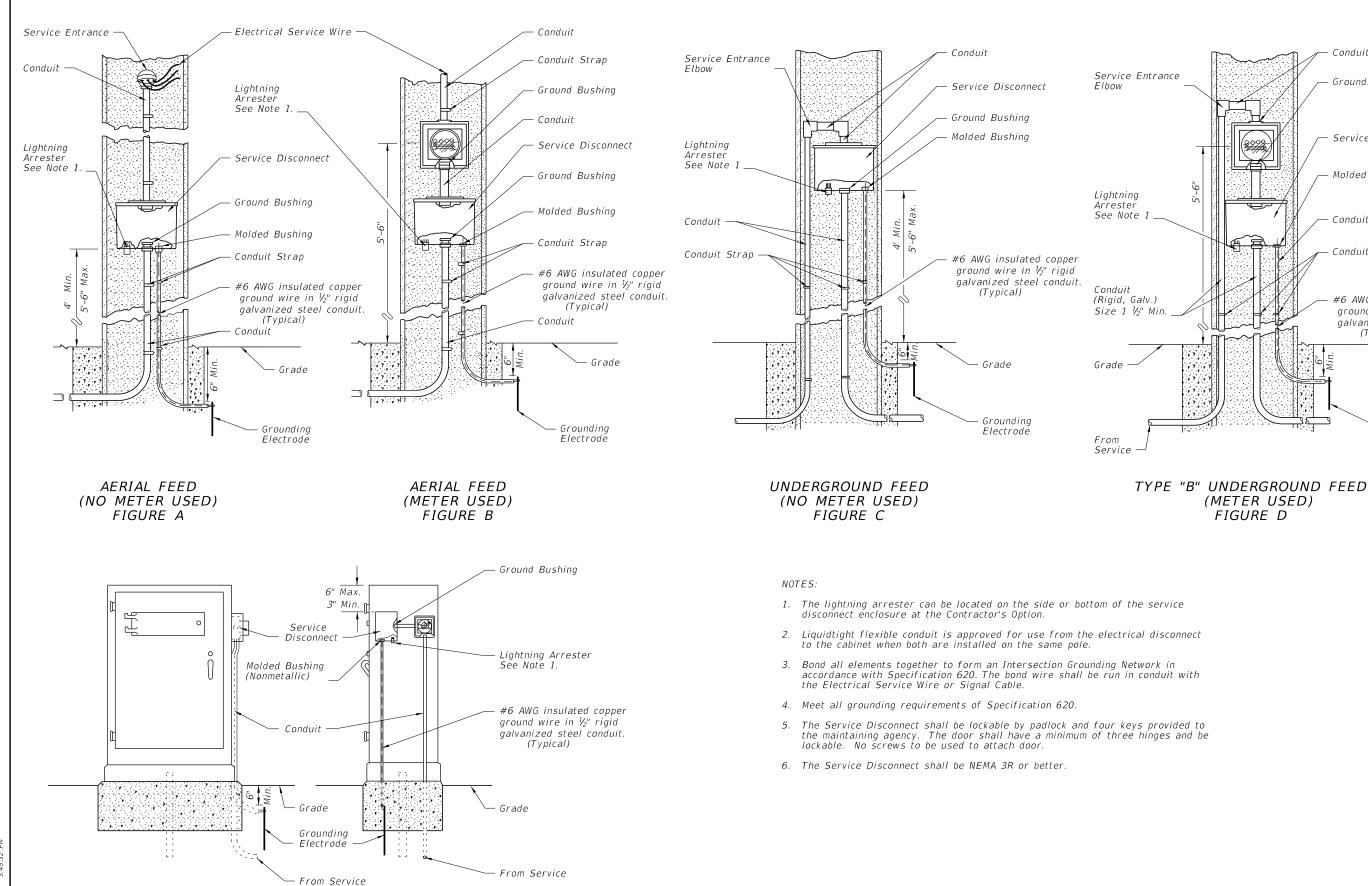
DESCRIPTION:

**FDOT** 

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SERVICE POINT DETAILS

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**REVISION** 11/01/17

DESCRIPTION:

FDOT

UNDERGROUND CABINET MOUNTED (METER USED) FIGURE E

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Conduit

Grounding Bushing

Service Disconnect

Molded Bushing

Conduit Strap

#6 AWG insulated copper

ground wire in ½" rigid

galvanized steel conduit.

Grounding

Electrode

(Typical)

Conduit

SHEET

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#### GENERAL NOTES:

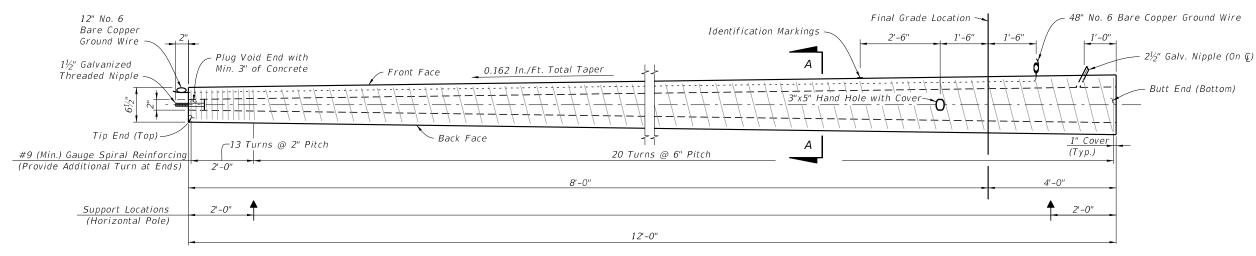
- Work these Index drawings with the Strain Pole Schedule in the Plans. Shop Drawings: This Index is considered fully detailed and no shop drawings are necessary. Submit shop drawings for minor modifications not detailed in the plans.
- Materials:
  - Concrete: Class V Special or Class VI
    Prestress Strands & Spiral Reinforcing: Specification Section 641
    Hand and coupler cover plates:
  - Hand and coupler cover plates: Non-corrosive material Screws: Round headed, chrome plated
- 4. Fabrication:
  - A. Pole Taper for pole width, strands, reinforcing and void: 0.081 in/ft per face. B. Concrete Cover: 1" minimum

  - Spiral Reinforcing: As shown, plus one turn for splices and two turns at both the tip and butt ends
  - The design dimensions for Front Face (FF) and Back Face (BF) of the poles may vary transversely from the section shown by  $\pm \frac{1}{4}$ " to assist with removal from forms. Balance addition and subtraction of the face widths to maintain section areas shown.
  - Tie ground wires to the interior of reinforcing steel to prevent displacement during concreting operations.
  - Cut the tip end of the prestressed strand first or simultaneously with the butt end
  - Provide cover plates and screws for hand hole and couplers. Attach cover plates to the poles using lead anchors or embedded threaded inserts.
  - Provide Aluminum Identification Tags on the poles with the following information:
    - Financial Project ID.
    - Pole Manufacturer
    - Standard Pole Type Number
  - d. Pole Length (L)
- Support locations are for strand release, storage, lifting and transport. Keep BF oriented downward until final erection.
- Pick-up and support locations shown may vary within a tolerance of  $\pm 3$ ".
- 7. Two point attachment: provide an eye bolt hole for the messenger wire. 8. Tether Wire: When required, field-drill the eyebolt hole prior to installation

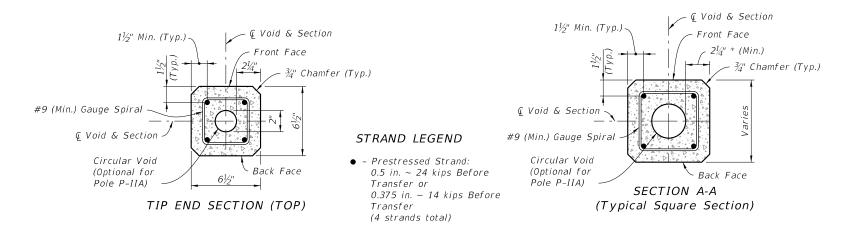
DESCRIPTION: LAST **REVISION** 11/01/17

FDOT

# SERVICE POLE P-IIA (12 Ft.) & P-IIB (36 Ft.) ELEVATION (Strands Not Shown)



# PEDESTAL POLE P-IIC (12 Ft.) ELEVATION (Strands Not Shown)



#### NOTES:

Strands shown are continuous from Tip End to Butt End.

Elevation view scale is exaggerated vertically for clarity.

For final erection, tilt pole upright with single point attachment located a distance of 4 Ft. (for P-IIA & P-IIC) or 10 Ft. (for P-IIB) from the Tip End.

* Dimension may vary from  $2\frac{1}{4}$ " to  $3\frac{1}{2}$ " to accommodate smaller radius of optional stepped (PVC) void. The void diameter shall not be less than 2".

## SERVICE AND PEDESTAL POLE TYPE P-II

LAST REVISION 11/01/17

DESCRIPTION:

F

FDOT STA

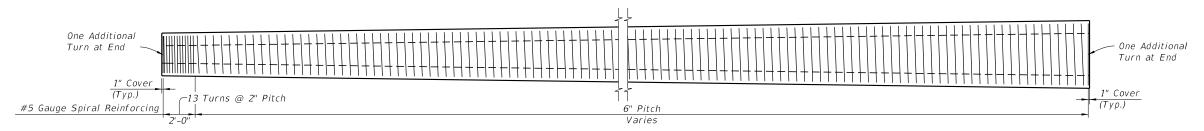
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CONCRETE POLES

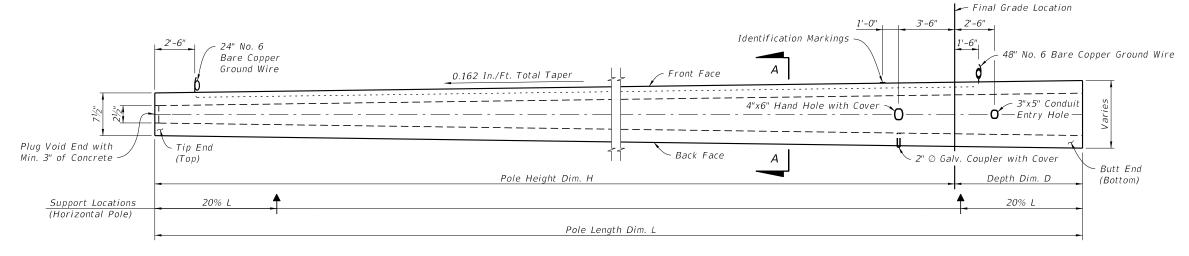
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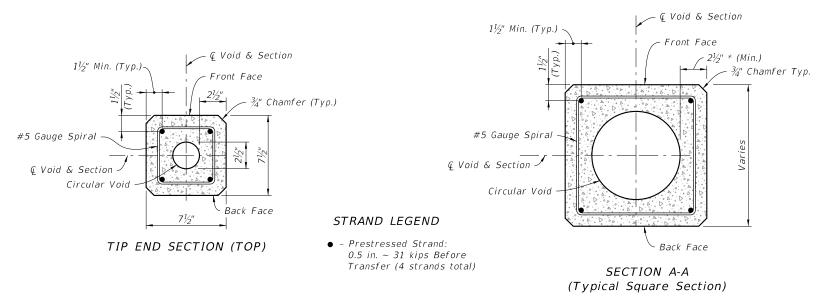
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#### SPIRAL REINFORCING ELEVATION (Strands, Holes, and Fixtures Not Shown)



#### POLE ELEVATION (Strands and Reinforcing Not Shown)



#### NOTES:

Strands shown are continuous from Tip End to Butt End.

Elevation view scale is exaggerated vertically for clarity.

For final erection, tilt pole upright with single point attachment located a distance 33.3% L from Tip End.

* Dimension may vary from  $2\frac{1}{2}$ " to  $3\frac{3}{4}$ " to accommodate smaller radius of optional stepped (PVC) void. The void diameter shall not be less than  $2\frac{1}{2}$ ".

POLE TYPE P-III

**REVISION** 11/01/17

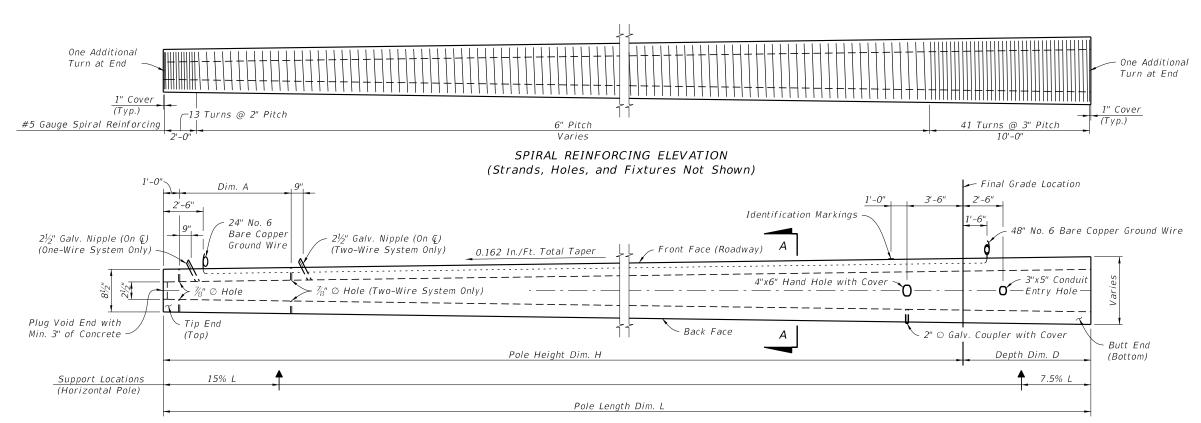
DESCRIPTION:

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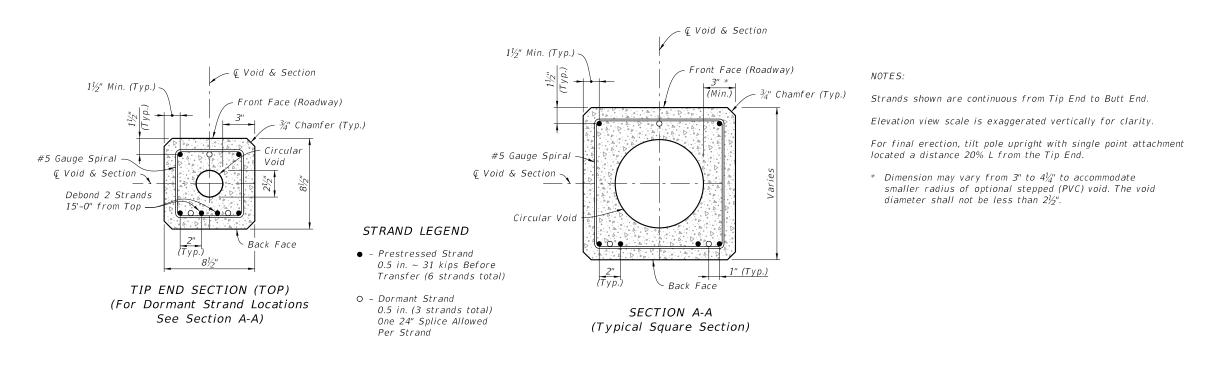
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CONCRETE POLES

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STRAIN POLE TYPE P-IV

LAST **REVISION** 11/01/17

FDOT

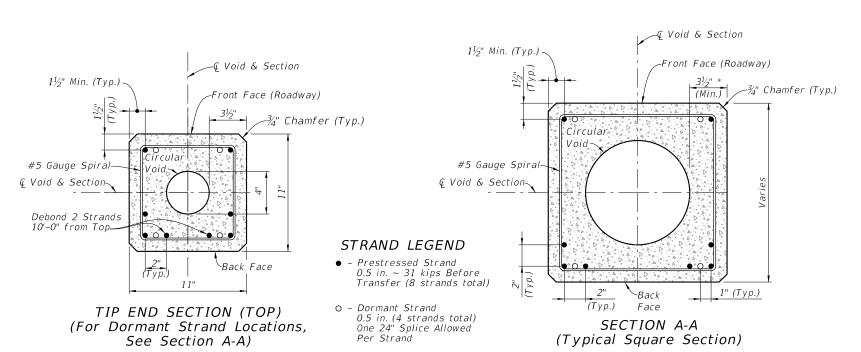
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CONCRETE POLES

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#### NOTES:

Strands shown are continuous from Tip End to Butt End.

Elevation view scale is exaggerated vertically for clarity.

For final erection, tilt pole upright with single point attachment located a distance 12.5% L from the Tip End.

* Dimension may vary from  $3\frac{1}{2}$ " to  $4\frac{3}{4}$ " to accommodate smaller radius of optional stepped (PVC) void. The void diameter shall not be less than 4".

STRAIN POLE TYPE P-V

LAST **REVISION** 11/01/17

DESCRIPTION:

FDOT

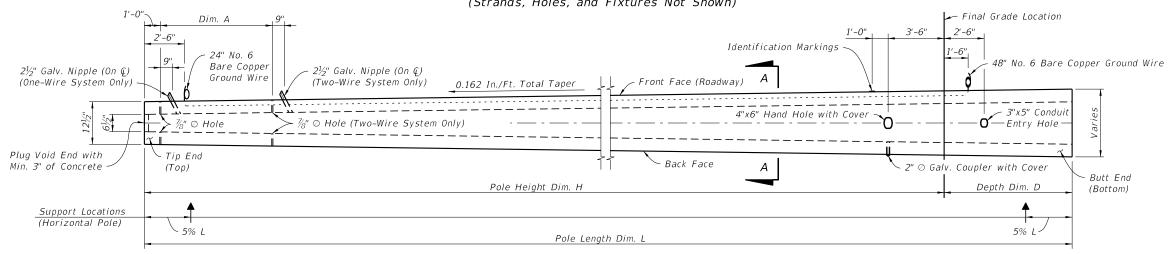
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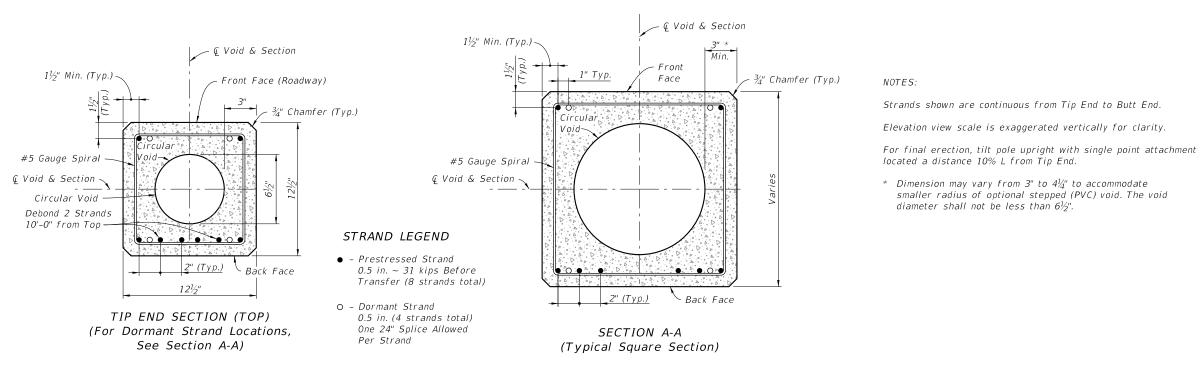
CONCRETE POLES

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## STRAIN POLE TYPE P-VI

**REVISION** 11/01/17

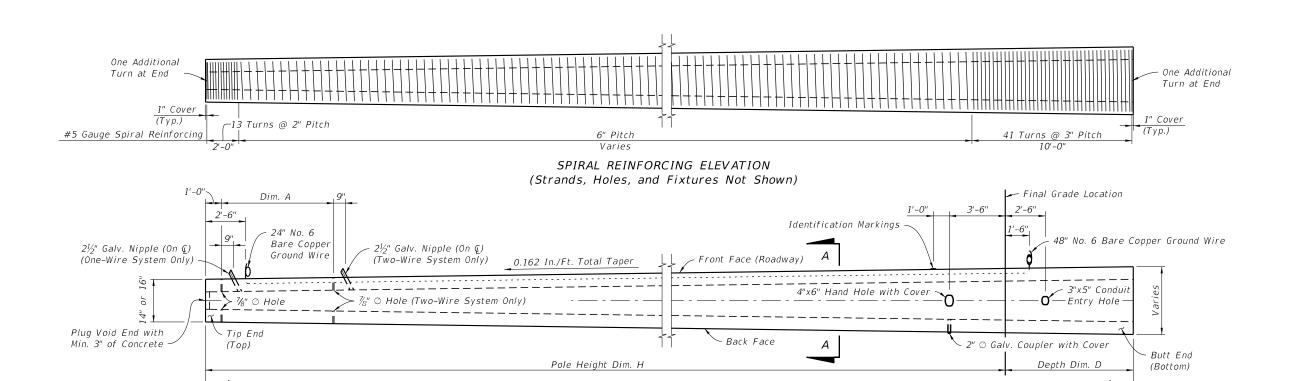
DESCRIPTION:

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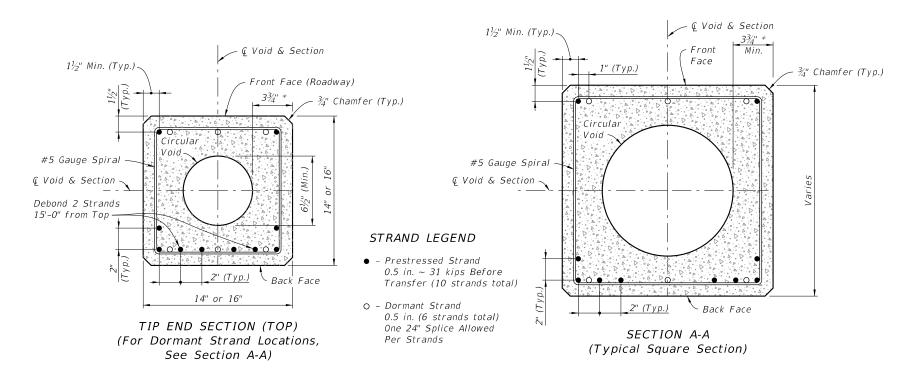
FY 2019-20 STANDARD PLANS

CONCRETE POLES

INDEX 641-010



Pole Length Dim.



#### NOTES:

≤ 2.5% L

Strands shown are continuous from Tip End to Butt End.

Elevation view scale is exaggerated vertically for clarity.

For final erection, tilt pole upright with single point attachment located a distance 10% L from the Tip End.

* Dimension may vary from 3¾" to 5" to accommodate smaller radius of optional stepped (PVC) void. The void diameter shall not be less than 6½".

STRAIN POLE TYPE P-VII

LAST REVISION 11/01/17

FDOT

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STANDARD PLANS

CONCRETE POLES

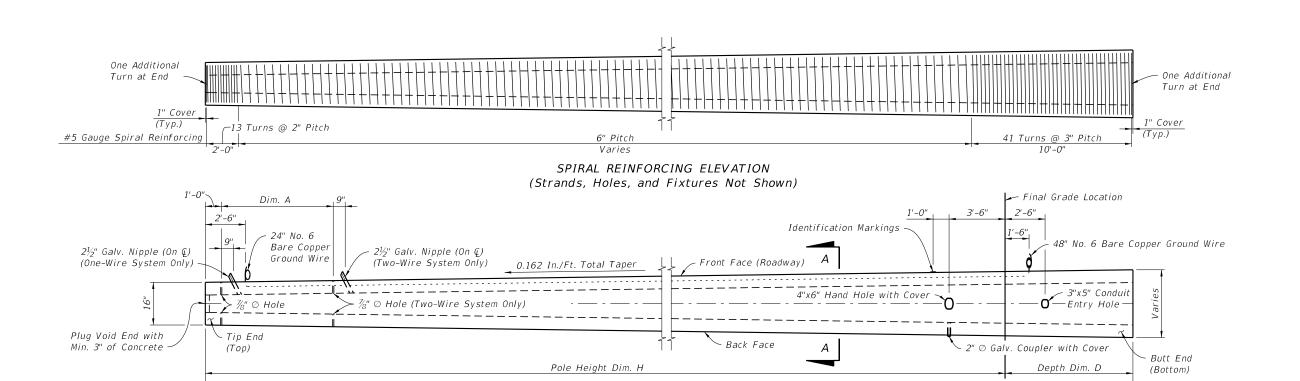
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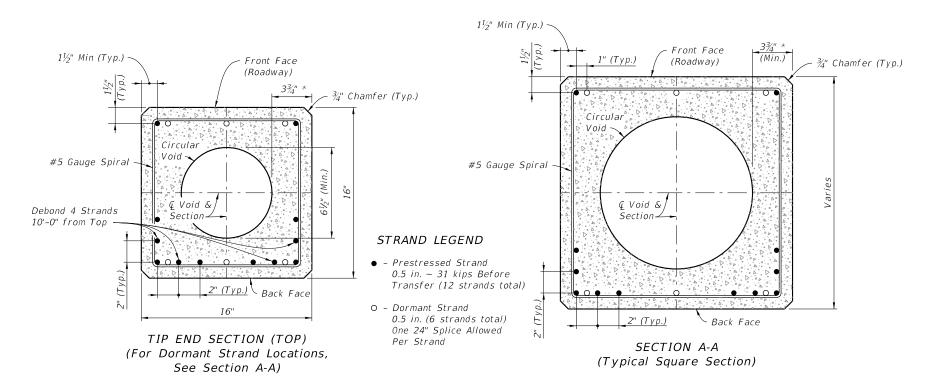
E POLES 641-010

Support Locations (Horizontal Pole)

≤ 2.5% L



Pole Length Dim. I



#### NOTES:

≤ 2.5% L

Strands shown are continuous from Tip End to Butt End.

Elevation view scale is exaggerated vertically for clarity.

For final erection, tilt pole upright with single point attachment located a distance 10% L from the Tip End.

* Dimension may vary from  $3\frac{3}{4}$ " to 5" to accommodate smaller radius of optional stepped (PVC) void. The void diameter shall not be less than  $6\frac{1}{2}$ ".

STRAIN POLE TYPE P-VIII

**REVISION** 11/01/17

DESCRIPTION:

Support Locations (Horizontal Pole)

≤ 2.5% L

FDOT

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CONCRETE POLES

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#### **GENERAL NOTES:**

- 1. Work this Index with Specifications 641.
- 2. This Index is considered fully detailed and no shop drawings are necessary. Submit Shop Drawings for minor modifications not detailed in the Plans.
- 3. Install pole plumb
- 4. Provide either round or 12-sided Poles.
- 5. See Index 635-001 for additional details for Pull Boxes.
- 6. Materials:
  - A. Pole: Use Class VI concrete with 6 ksi minimum strength at transfer.
- B. Prestressing Strands: ASTM A416, Grade 270 low relaxation.
- C. Reinforcing Steel: ASTM A615, Grade 60
- D. Spiral Reinforcing: ASTM A1064 Cold-Drawn
- E. Bolts: ASTM F1554, Grade 55
- Nuts: ASTM A563, Grade A Heavy Hex
- Washers: ASTM F436
- F. Steel plates and Pole Cap: ASTM A36 or ASTM A709, Grade 50
- G. Galvanization: Bolts, nuts and washers: ASTM F2329 All other steel: ASTM A123

#### 7. Pole Fabrication:

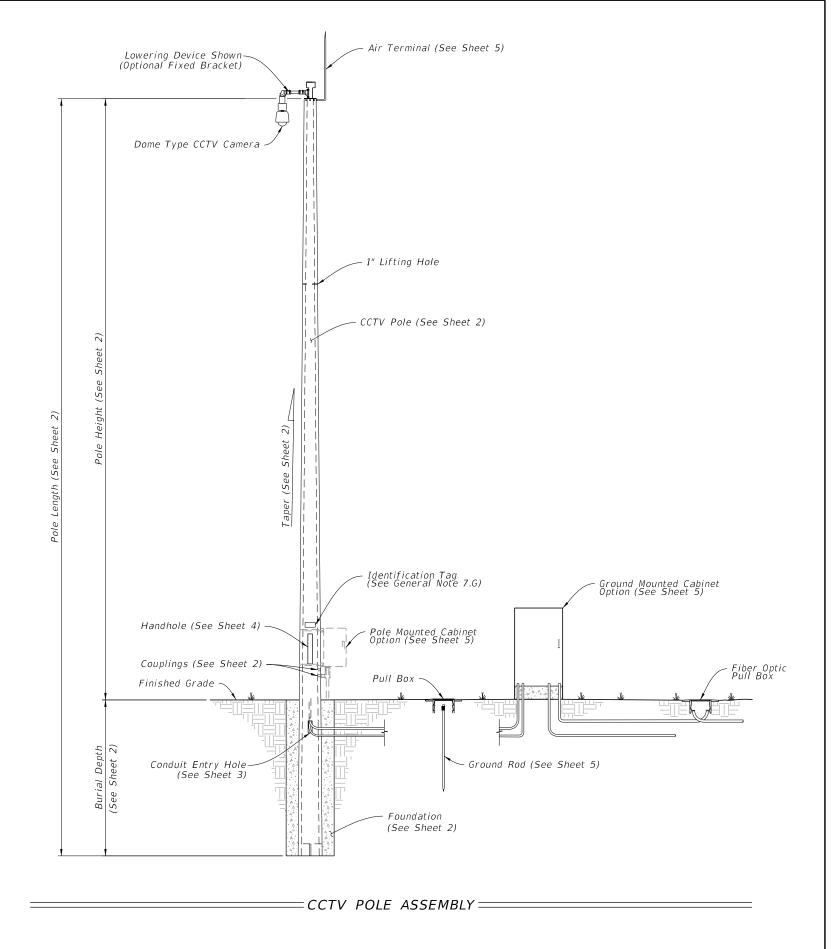
- A. Cut the tip end of the prestressed strand first or simultaneously with the butt end.
- B. For spiral reinforcing, one turn is required for spiral splices and two turns are required at the top and bottom of poles.
- C. For Reinforcing Steel, lap splice to consist of a 3'-0" lap length at each splice. No more
- than two opposing rebar to be spliced at the same cross section. Stagger lap splices as needed.
- D. Provided a Class 3 surface finish in accordance with Specification 400.
- E. Provide a 1" minimum cover.
- F. Provide handhole and coupler cover plates made of non-corrosive materials. Attach cover plates to poles using lead anchors or threaded inserts embedded in the poles in conjunction with round headed chrome plated screws.
- G. Provide Identification Markings on the poles where indicated on the following sheets. Include the following information using inset numerals with 1" height or as approved in the Producers' Quality Control Program:

Financial Project ID Pole Manufacturer Pole Length

- H. Tie ground wires to the interior of reinforcing steel as necessary to prevent displacement during concreting operations.
- I. Storage, Handling and Erection locations shown may vary within  $\pm 3$ ".

#### 8. Cabinet Installation:

- A. Splice fiber optic cables in cabinet to preterminater patch panel.
- B. Furnish and install Surge Protection Devices (SPDs) on all cabling in cabinet.
- C. Furnish and install secondary SPDs protection on outlets for equipment in cabinet
- D. Ensure that all electronic equipment power is protected and conditioned with SPDs. E. Ensure that equipment cabinet is bonded to CCTV pole grounding system.
- F. Install the pole mounted cabinet with the hinges next to the pole.
- G. Sizes and types of conduits and innerducts for network communications between the pullbox and cabinet are stated in the Contract Documents.
- 9. Lowering Device Installation:
  - A. Place the lowering cable that moves within the pole in an interior conduit to prevent it from tangling or interfering with any electrical wire that is in the pole. Ensure that any electrical wire within the pole is routed securely and free from slack.
- B. Mount lowering arm perpendicular to the roadway or as shown in the plans. Position CCTV pole so that the camera can be safely lowered without requiring lane closures.
- C. Coordinate all lowering device hardware requirements (including Tenon, Tenon mounting plates, parking stand, etc.) with lowering device manufacturer.



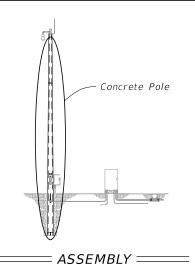
**REVISION** 11/01/17

DESCRIPTION:



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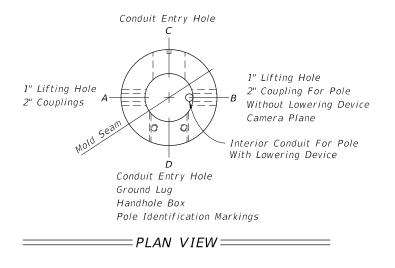
### NOTES:

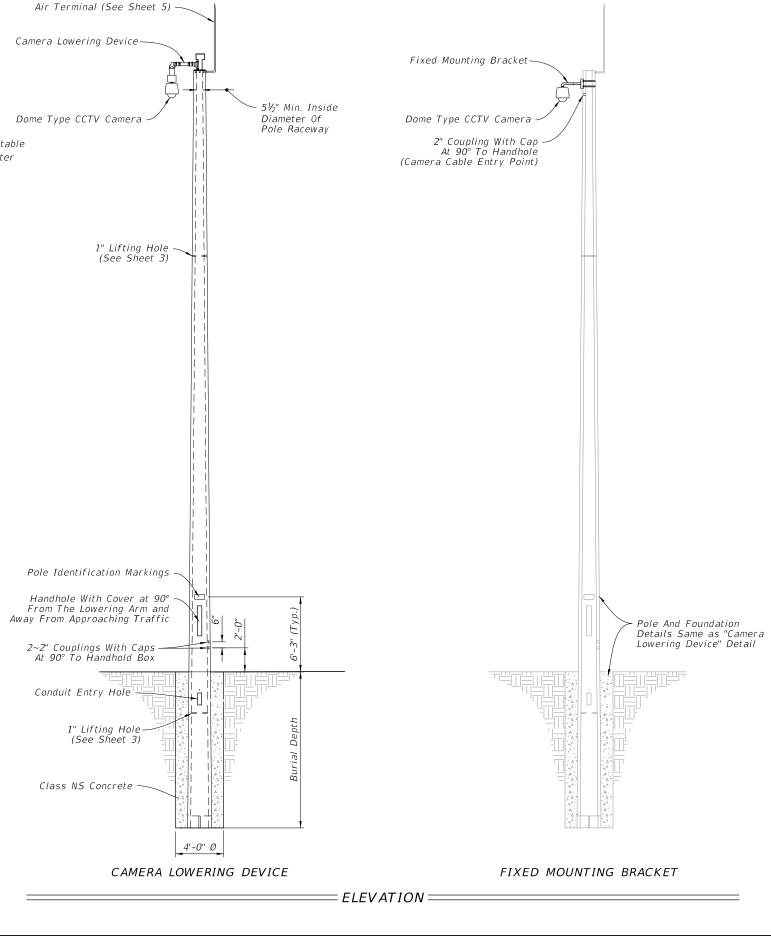
- 1. Diameter of 12-sided poles are measured flat to flat.
- 2. Total Taper applies to pole, strands and reinforcing.
- 3. For 12-Sided Pole and Round Roles Option 2, Stress prestressed strand to 70% of Ultimate before transfer. For Round Pole Option 1, stress prestressed strand to 60% of Ultimate before transfer.
- 4. Pole Design Tables, Burial Depth is based on level ground (flatter than 1:5). Increase the burial depth in accordance with the Additional Burial Depth Due To Ground Slope table for foundations with slopes 1:5 and steeper. Use the higher value for slope or diameter values that fall between those shown on the table.

ADDITIONAL BURIAL DEPTH DUE TO GROUND SLOPE							
Ground Slope	Additional Burial Depth (feet)						
1:5	3						
1:4	4						
1:3	5						
1:2	7						

			12	-SIDEL	POLE	DESIGN	V TABL	<b>E</b> (See N	ote 1)	
Pole Length (ft)	Pole Height (ft)	Burial Depth (ft)	l laner	Void Taper (in/ft)	Min. Wall Thickness Tip (in)	Thicknoon	Tip Diameter (in)	Butt Diameter (in)	Strand Pattern	Strand Diameter
63	50	13	0.18	0.18	3	3	12	23.34	1	0.6"
69	55	14	0.18	0.18	3	3	12	24.42	1	0.6"
75	60	15	0.18	0.18	3	3	12	25.50	2	0.6"
80	65	15	0.18	0.18	3	3	12	26.40	2	0.6"
86	70	16	0.18	0.18	3	3	12	27.48	2	0.6"

				ROUI	VD P	OLE DE	SIGN T	ABLE			
Pole Length (ft)	Pole Height (ft)	Burial Depth (ft)	Design Option	Total Taper (in/ft) (See Note 2)	Void Taper (in/ft)	Min. Wall Thickness Tip (in)	Min. Wall Thickness Butt (in)	Tip Diameter (in)	Butt Diameter (in)	Strand Pattern	Strand Diameter
63	50	13	Option 1	0.216	0.192	3	3.76	12.15	25.76	3	0.5"
03	30	15	Option 2	0.180	0.172	3	3.50	12.00	23.34	4	0.5"
69	55	14	Option 1	0.216	0.192	3	3.83	12.15	27.05	3	0.5"
09	55	14	Option 2	0.180	0.173	3	3.50	12.00	24.42	4	0.5"
7.5	60	15	Option 1	0.216	0.192	3	3.90	12.15	28.35	3	0.5"
/3	60	15	Option 2	0.180	0.173	3	3.50	12.00	25.50	4	0.5"
80	65	15	Option 1	0.216	0.192	3	3.96	12.15	29.43	3	0.5"
00	05	15	Option 2	0.180	0.174	3	3.50	12.00	26.40	4	0.5"
86	86 70	16	Option 1	0.216	0.192	3	4.03	12.15	30.73	3	0.5"
	///	10	Option 2	0.180	0.174	3	3.50	13.00	28.48	4	0.5"

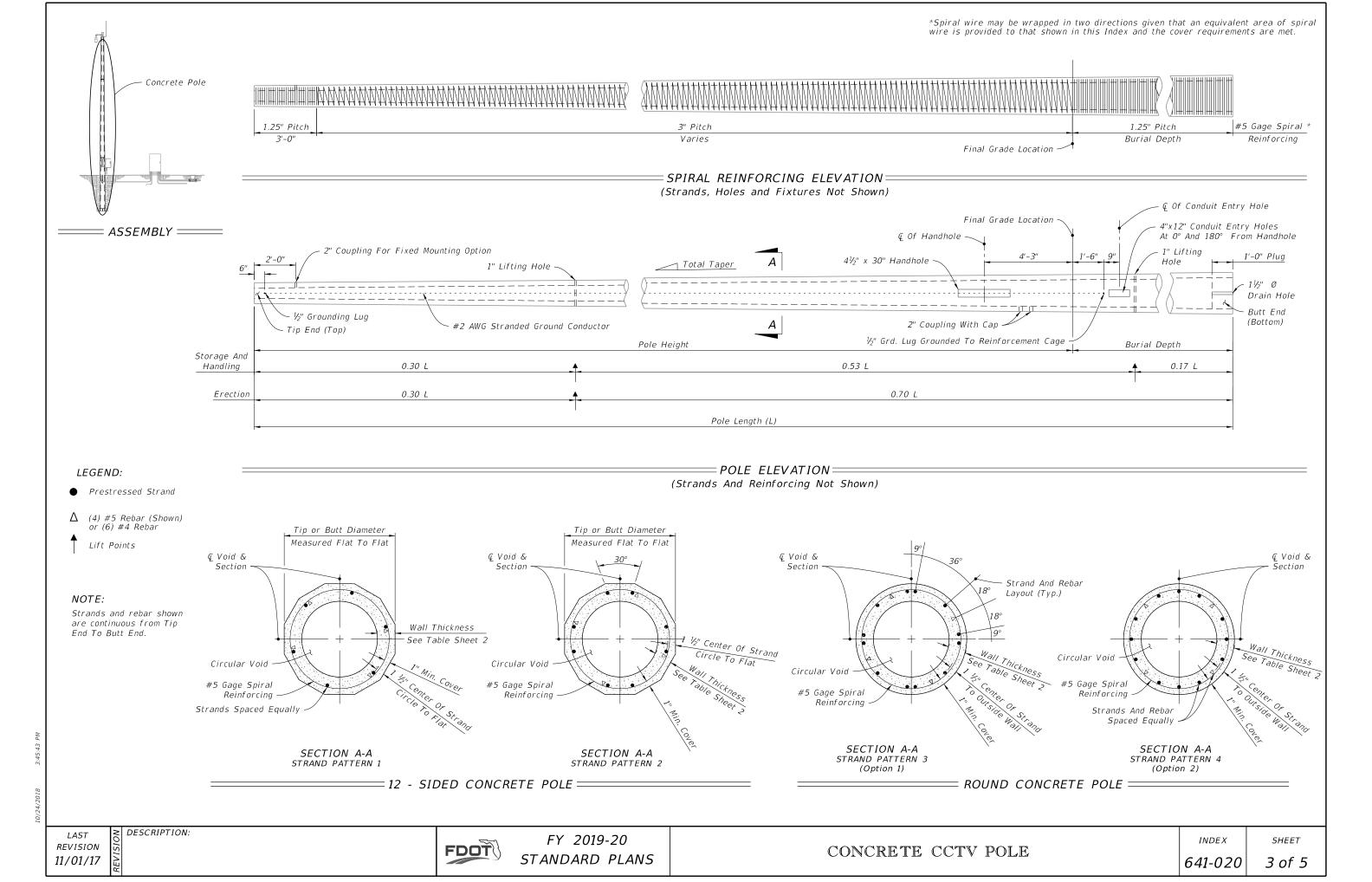


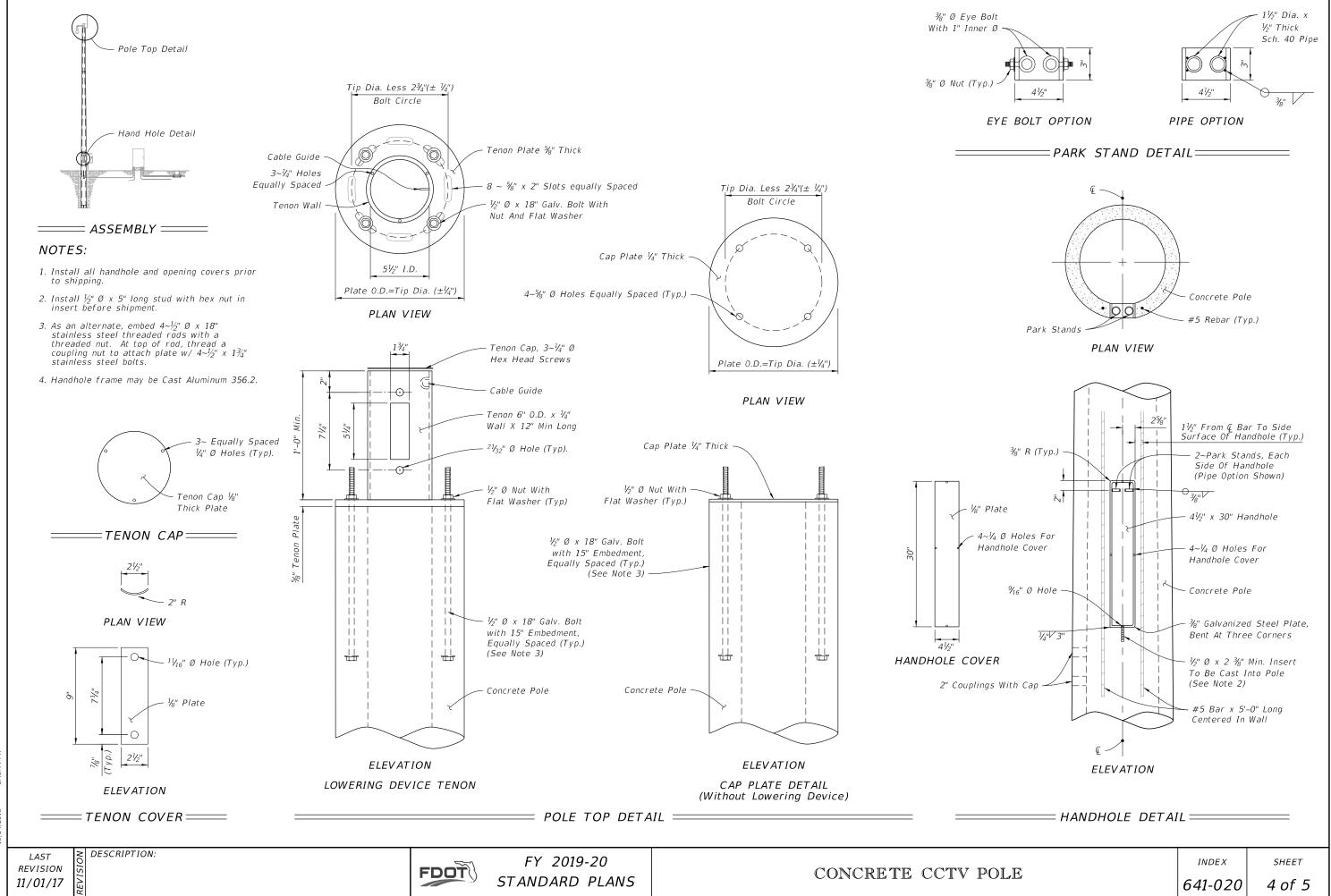


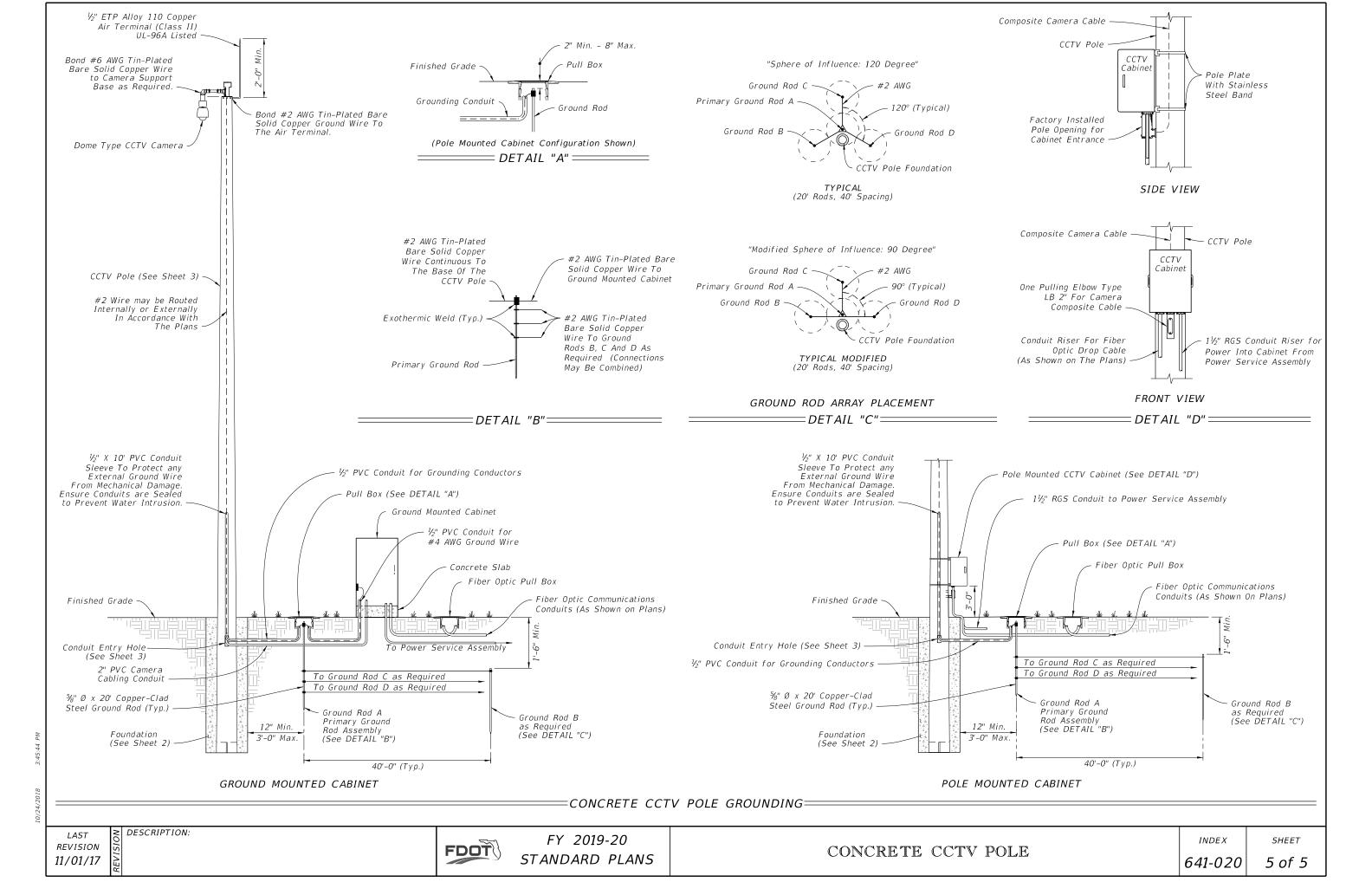
LAST REVISION 11/01/17

DESCRIPTION:









#### **NOTES:**

- 1. Work with Index 634-001 for grounding and span wire details. See the Plans for clamp spacing, cable sizes and forces, signals and sign mounting locations and details.

This Index is considered fully detailed, only submit shop drawings for minor modifications not detailed in the Plans.

- A. Strain Pole and Backing Rings:
  - a. Less than ¾₁₆": ASTM A1011 Grade 50, 55, 60 or 65
  - b. Greater than or equal to  $\frac{3}{16}$ ": ASTM A572 Grade 50, 55, 60 or 65
  - c. ASTM A595 Grade A (55 ksi yield) or Grade B (60 ksi yield)
- B. Steel Plates: ASTM A36
- C. Weld Metal: E70XX
- D. Bolts, Nuts and Washers:
  - a. High Strength Bolts: ASTM F3125, Grade A325, Type 1
  - b. Nuts: ASTM A563 Grade DH Heavy-Hex
- c. Washers: ASTM F436 Type 1, one under turned element
- E. Anchor Bolts, Nuts and Washers.
  - a. Anchor Bolts: ASTM F1554 Grade 55
  - b. Nuts: ASTM A563 Grade A Heavy-Hex (5 per anchor bolt)
  - c. Plate Washers: ASTM A36 (2 per bolt). Split-lock washers and self-locking nuts are not permitted
- F. Handhole Frame: ASTM A709 or ASTM A36, Grade 36
- G. Handhole Cover: ASTM A1011 Grade 50, 55, 60 or 65
- H. Aluminum Pole Caps and Nut Covers: ASTM B26 (319-F)
- I. Stainless Steel Screws: AISI Type 316
- J. Threaded Bars/Studs: ASTM A36 or ASTM A307
- K. Concrete: Class IV (Drilled Shaft) for all environmental classifications.
- L. Reinforcing Steel: Specification 415

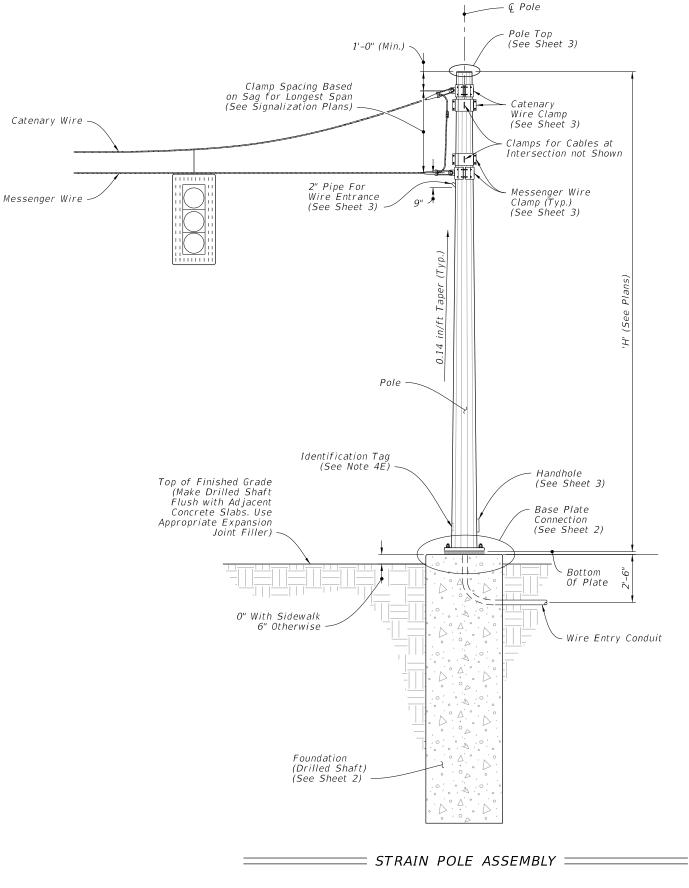
#### 4. Fabrication:

- A. Pole Taper: Change diameter at a rate of 0.14 inches per foot, round or 12-sided (Min.)
- B. Upright splices are not permitted. Transverse welds are only permitted at the base.
- C. Provide bolt hole diameters as follows:
  - a. Bolts (except Anchor Bolts): Bolt diameter plus  $V_{16}$ ", prior to galvanizing.
  - b. Anchor Bolts: Bolt diameter plus 1/2", maximum.
- D. Locate handhole 180° from 2" wire entrance pipe.
- E. Identification Tag: (Submit details for approval.)
  - a. 2"x 4" (Max.) aluminum identification tag.
  - b. Locate on the inside of the pole and visible from the handhole.
  - c. Secure to pole with  $\frac{1}{8}$ " diameter stainless steel rivets or screws.
  - d. Include the following information on the ID Tag:
    - 1. Financial Project ID
    - 2. Pole Type 3. Pole height

    - 4. Manufacturers' Name
    - 5. Fy of Steel
- 6. Base Wall Thickness
  F. Provide a 'J' or 'C' hook at the top of the pole for signal wiring support (See Sheet 3).
  G. Perform all welding in accordance with Specification 460–6.4.
  H. Hot Dip Galvanize after fabrication.

#### 5. Coatings:

- A. All Nuts, Bolts, Washers and Threaded Bars/Studs: ASTM F2329 B. All other steel items including plate washers: ASTM A123
- 6. Construction:
  - A. Foundation: Specification 455, except that payment is included in the cost of the strain pole.
  - B. After installation, place wire screen between top of foundation and bottom of baseplate in accordance with Specification 649-6.



ELEVATION AND NOTES

**REVISION** 11/01/17

DESCRIPTION:

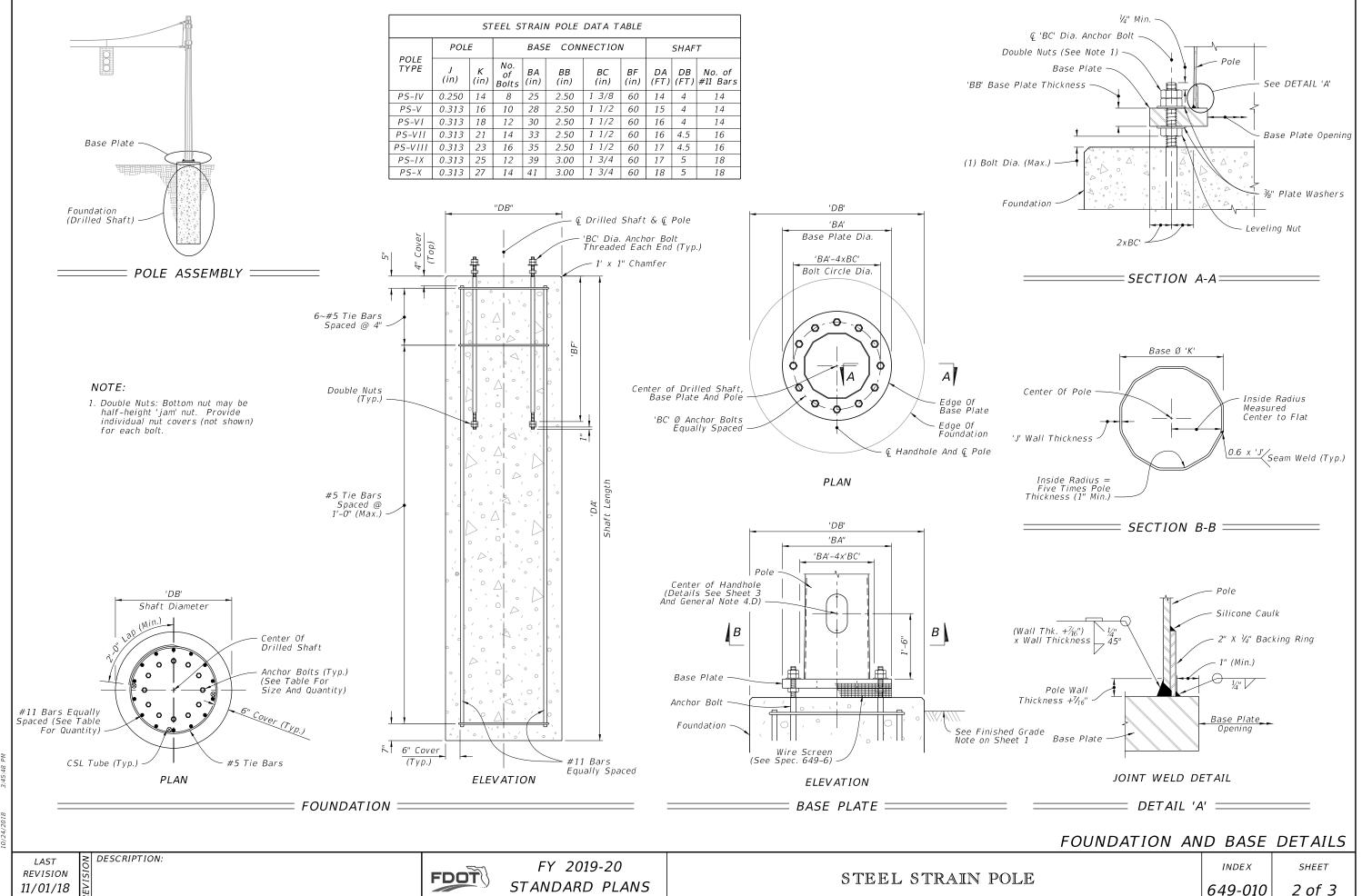
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FY 2019-20 STANDARD PLANS

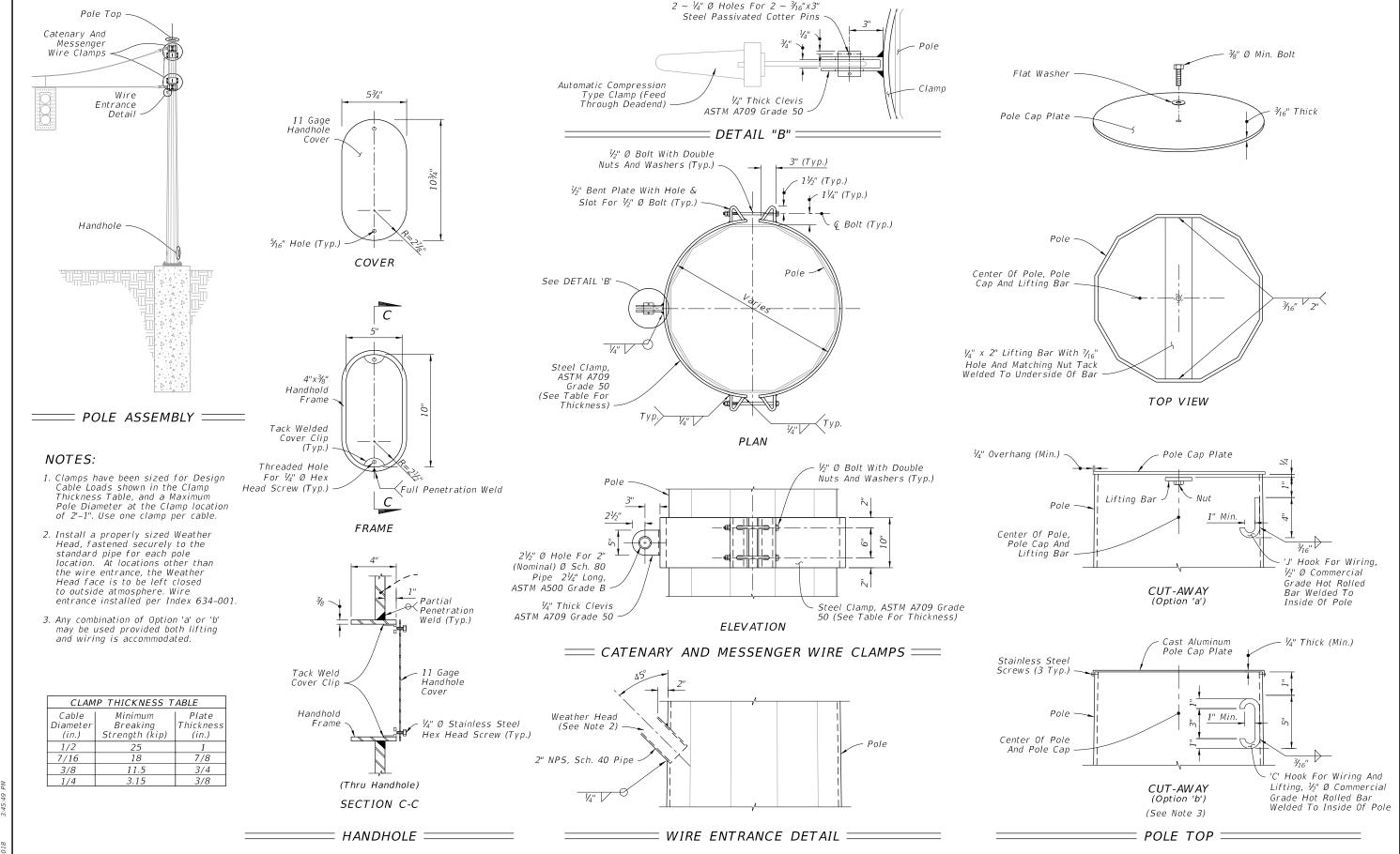
STEEL STRAIN POLE

*INDEX* 649-010

SHEET 1 of 3



2 of 3



**REVISION** 11/01/17

DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

STEEL STRAIN POLE

INDEX 649-010

ATTACHMENT DETAILS

SHEET

3 of 3

#### GENERAL NOTES:

- 1. Work this Index with Specification 649.
- 2. This Index is considered fully detailed; only submit shop drawings for minor modifications not detailed in the Plans.

#### 3. Materials

- A. Pole: ASTM A1011 Grade 50, 55, 60 or 65 (less than  $V_4$ ") or ASTM A572 Grade 50, 60 or 65 (greater than or equal to  $V_4$ ") or ASTM A595 Grade A (55 ksi yield) or Grade B (60 ksi yield).
- B. Steel Plates and Pole Cap: ASTM A36 or ASTM A709 Grade 50.
- C. Weld Metal: E70XX.
- D. Bolts: ASTM F3125, Grade A325, Type 1.

Nuts: ASTM A563.

- Washers: ASTM F-436.
- E. Anchor Bolts: ASTM F1554 Grade 55 with ASTM A563 Grade A heavy-hex nuts and ASTM A36 plate washers.
- F. Handhole Frame: ASTM A709 Grade 36 or ASTM A36.
- G. Handhole Cover: ASTM A1011 Grade 50, 55, 60 or 65.
- H. Stainless Steel Screws: AISI Type 316.
- I. Reinforcing Steel: ASTM A615 Grade 60.
- J. Galvanization: Bolts, nuts and washers: ASTM F2329 All other steel including plate washer: ASTM A123
- K. Concrete: Class IV (Drilled Shaft) for all environment classifications.

#### 4. <u>Fabrication:</u>

- A. Welding:
- a. Specification Section 460-6.4 and
- b. AASHTO RFD Specification for Structural Supports for Highway Signs, Luminaires, and Traffic Signals Section 14.4.4.
- B. Poles:
- a. Round or 16-sided (Min.)
- b. Taper pole diameter at 0.14 inches per foot
- c. Fabricate Pole longitudinal seam welds (2 maximum) with 60 percent minimum penetration or fusion welds except as follows:
- 1. Use a full-penetration groove weld within 6 inches of the circumferential tube-to-plate connection and
- 2. Use full-penetration groove welds on the female end section of telescopic (i.e., slip type) field splices for a minimum length of one and one-half times the inside diameter of the female section plus 6 inches.
- d. Pole shaft may be either one or two sections (with telescopic field splice)
- e. Circumferentially welded pole shafts and laminated pole shafts are not permitted
- C. Identification Tag: (Submit details for approval)
- a. 2"x 4" (Max.) aluminum tag
- b. Locate on the inside of the pole and visible from the handhole
- c. Secure with 1/8" diameter stainless steel rivets or screws.
- d. Include the following information on the ID Tag:
  - 1. Financial Project ID
  - 2. Pole Type
  - 3. Pole Height
  - 4. Manufacturers' Name
  - 5. Yield Strength (Fy of Steel)
  - 6. Base Wall Thickness
- D. Except for Anchor Bolts, bolt hole diameters are bolt diameter plus  $\frac{1}{2}$  and anchor bolt holes are bolt diameter plus  $\frac{1}{2}$  (Max) prior to galvanizing.

#### 5. Pole Installation:

- $\overline{\text{A. Do not install}}$  additional wire access holes (not shown in this Index) with a diameter that exceeds  $1\frac{1}{2}$ " in diameter.
- B. Install Anchor Bolts in accordance with Specification 649-5
- C. Cable Supports: Electrical Cable Guides and Eyebolts.
- a. Locate top and bottom cable guides within the pole aligned with each other.
- b. Position one cable guide 2" below the handhole.
- c. Position other cable guide 1" directly below the top of the tenon.
- d. Position Park Stands 2" below the top of the handhole.

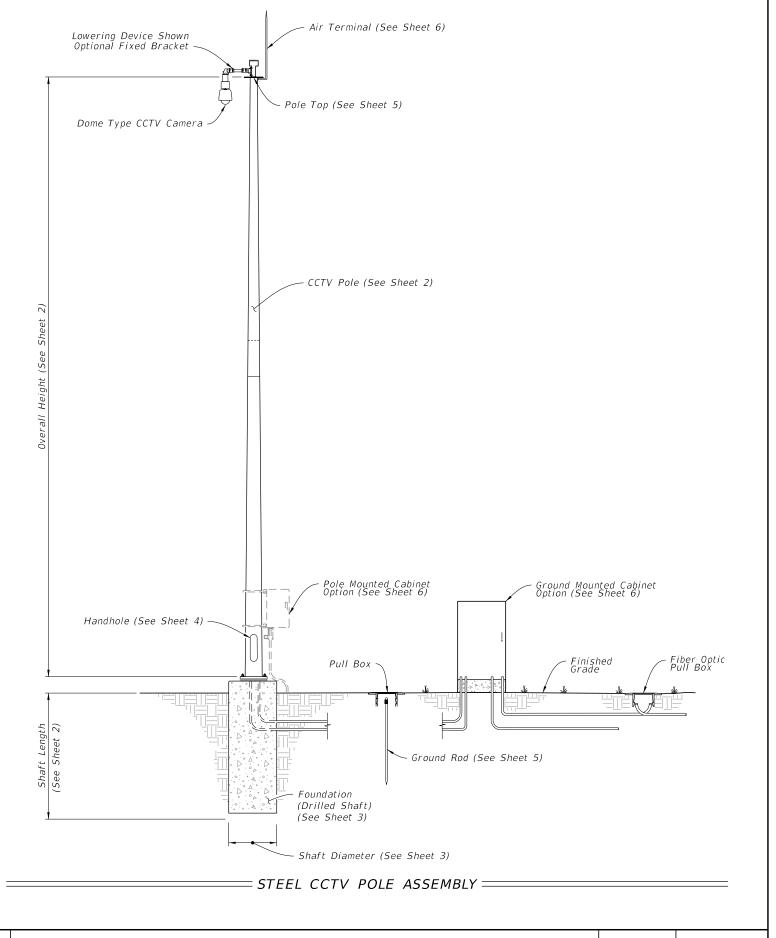
#### 6. Cabinet Installation:

- A. Splice fiber optic cables in cabinet to preterminater patch panel.
- B. Furnish and install Surge Protection Devices (SPDs) on all cabling in cabinet.
- C. Furnish and install secondary SPDs protection on outlets for equipment in cabinet.
- D. Ensure that all electronic equipment power is protected and conditioned with SPDs.
- E. Ensure that equipment cabinet is bonded to CCTV pole grounding system.
- F. Install the pole mounted cabinet with the hinges next to the pole.
- G. Sizes and types of conduits and inner ducts for network communications between the pullbox and cabinet are stated in the Contract Documents.

#### 7. Lowering Device Installation:

DESCRIPTION:

- A. Place the lowering cable that moves within the pole in an interior conduit to prevent it from tangling or interfering with any electrical wire that is in the pole. Ensure that any electrical wire within the pole is routed securely and free from slack.
- B. Mount lowering device perpendicular to the roadway or as shown in the plans. Position CC TV pole so that the camera can be safely lowered without requiring lane closures.
- C. Coordinate all lowering device hardware requirements (including Tenon, Tenon mounting plates, parking stands, etc.) with lowering device manufacturer.



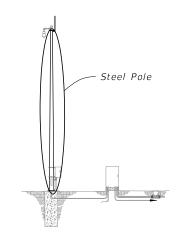
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FY 2019-20 STANDARD PLANS

STEEL CCTV POLE

INDEX



SHA	FT DE	SIGN T	ABLE
Pole Overall Height (ft)		Shaft Length	Longitudinal Reinforcement
50	4'-0"	11'-0"	(14) #11
55	4'-0"	12'-0"	(14) #11
60	4'-6"	13'-0"	(16) #11
65	4'-6"	13'-0"	(16) #11
70	5'-0"	14'-0"	(18) #11

=== ASSEMBLY =====

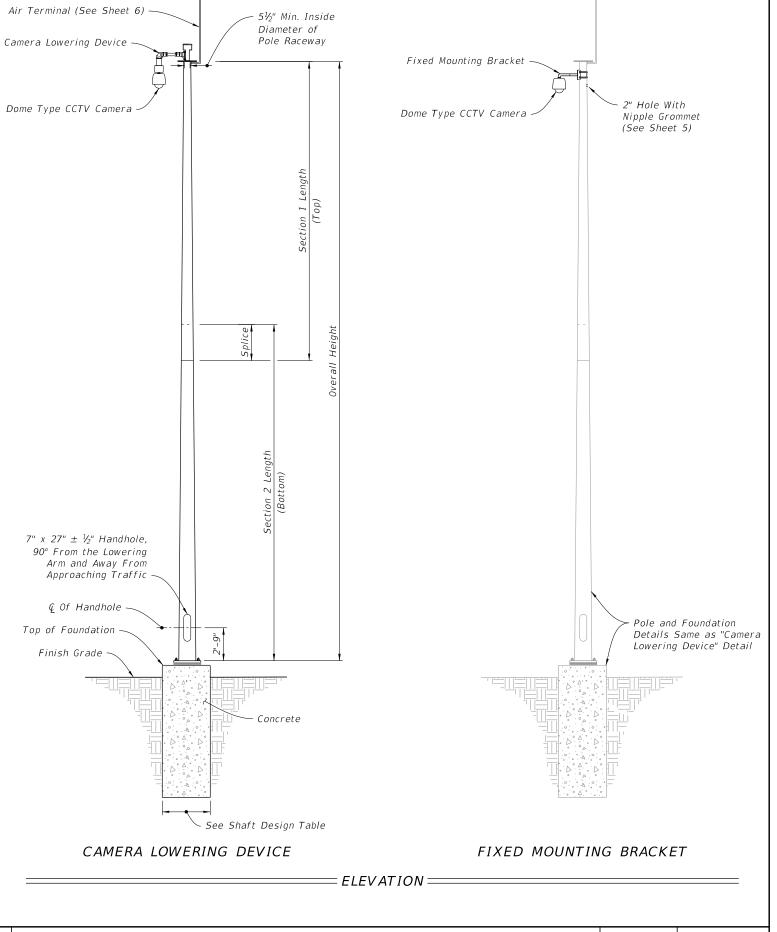
ADDITIO	ONAL SHAFT	DEPTH
DUE T	O GROUND	SLOPE
Ground Slope	4'-0" Shaft Diameter	5'-0" Shaft Diameter
1:5	3'-0"	4'-0"
1:4	4'-0"	5'-0"
1:3	5'-0"	6'-0"
1:2	7'-0"	9'-0"

#### **FOUNDATION NOTES:**

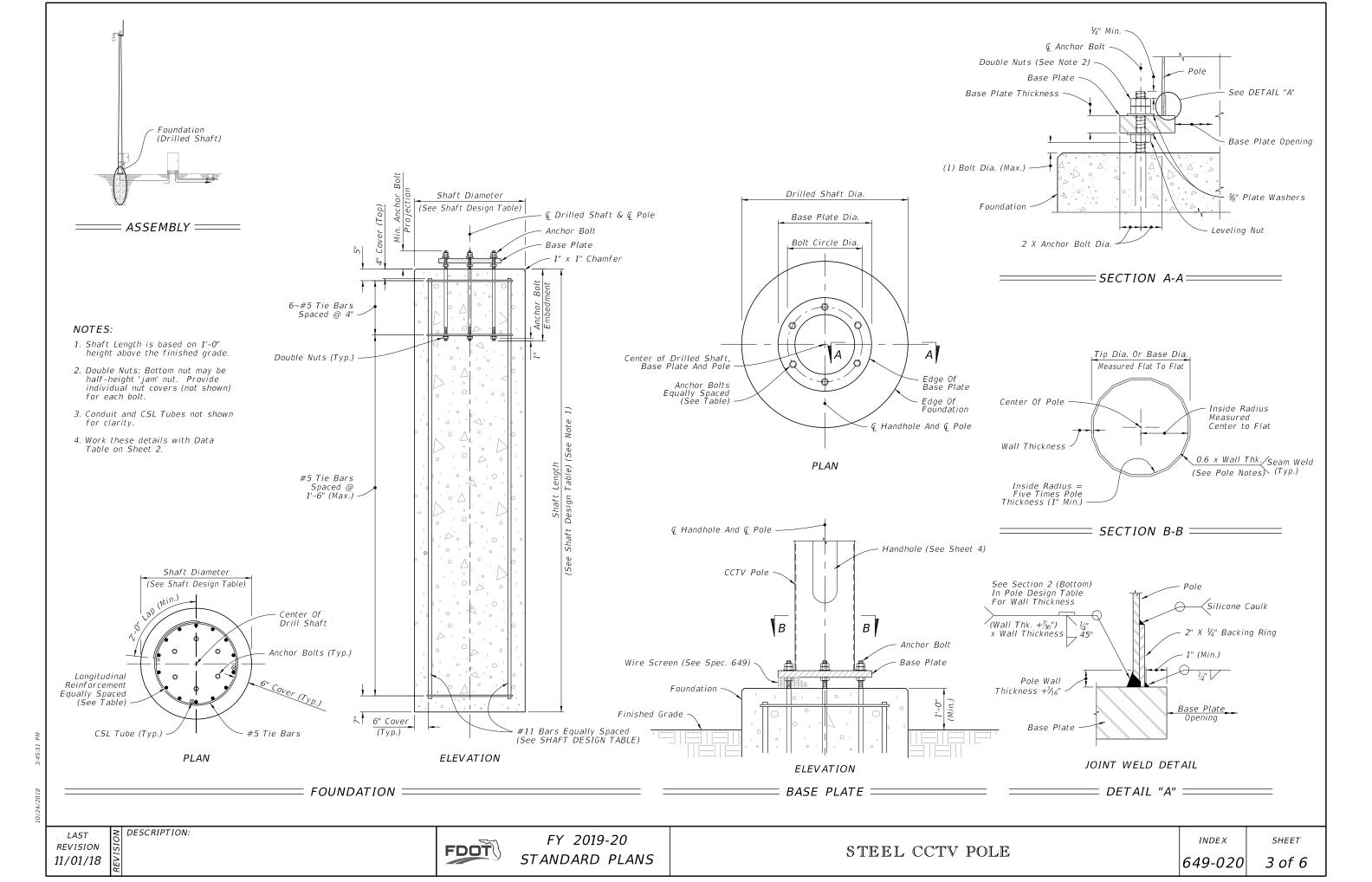
- 1. Shaft Length is based on 1'-0" height above the finished grade.
- 2. Shaft Design Table Shaft Length is based on level ground (flatter than 1:5). Increase the shaft depth in accordance with the Additional Shaft Depth Due To Ground Slope table for foundations with slopes 1:5 and steeper. Use the higher value for slope or diameter values that fall between those shown on the table.

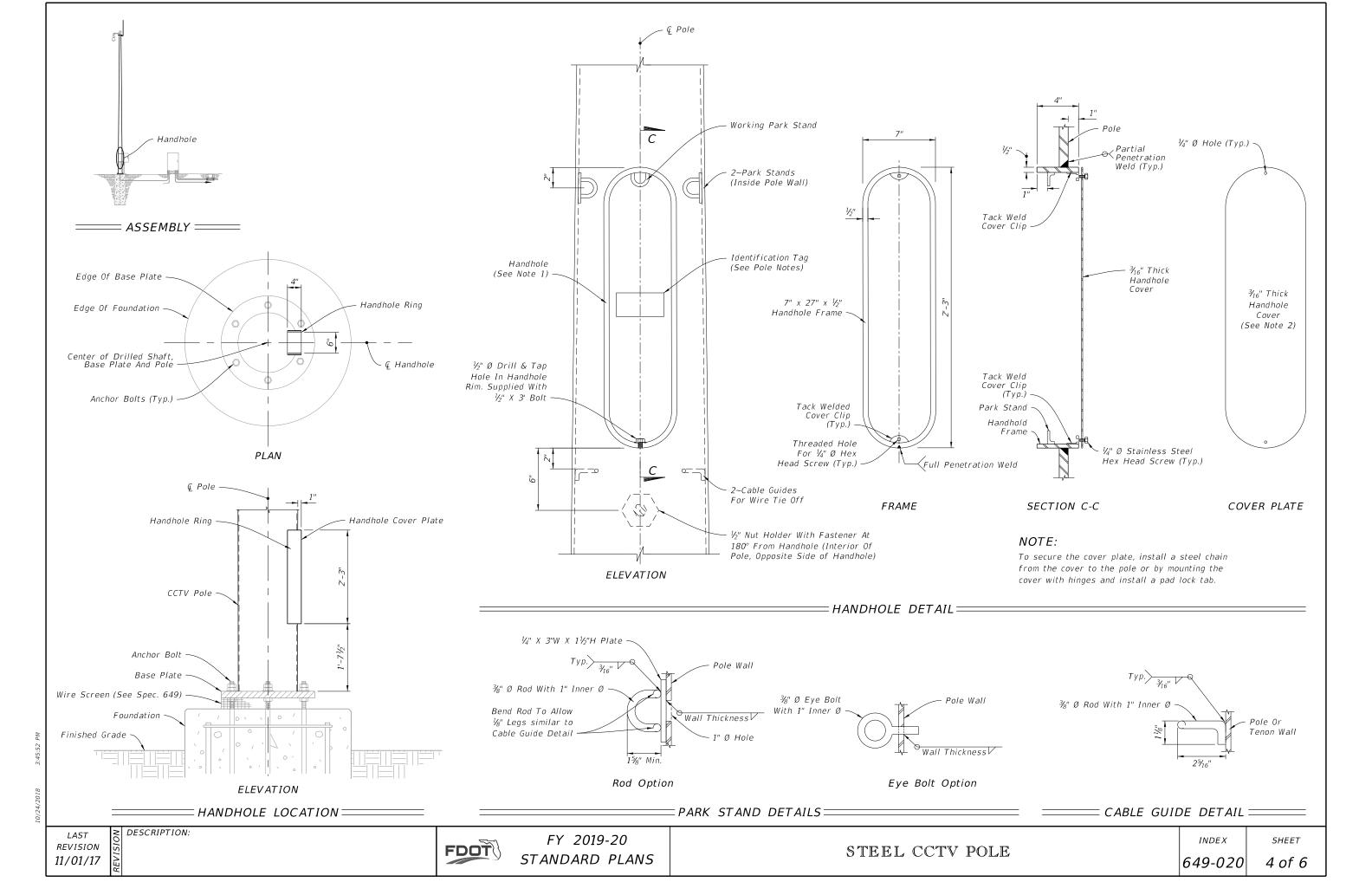
	BASE	PLATE A	ND ANCH	IOR BC	LT DESI	GN TABL	E
Pole Overall Height (ft)		Base Plate Thickness (in.)		Number of Bolts			Minimum Anchor Bolt Projection (in.)
50	27	2.5	22	6	1.25	31	8.5
55	28	2.5	23	6	1.25	33	8.5
60	33	2.5	27	6	1.50	34	9.5
65	35	2.5	29	6	1.50	35	9.5
70	40	2.5	33	6	1.75	38	10.5

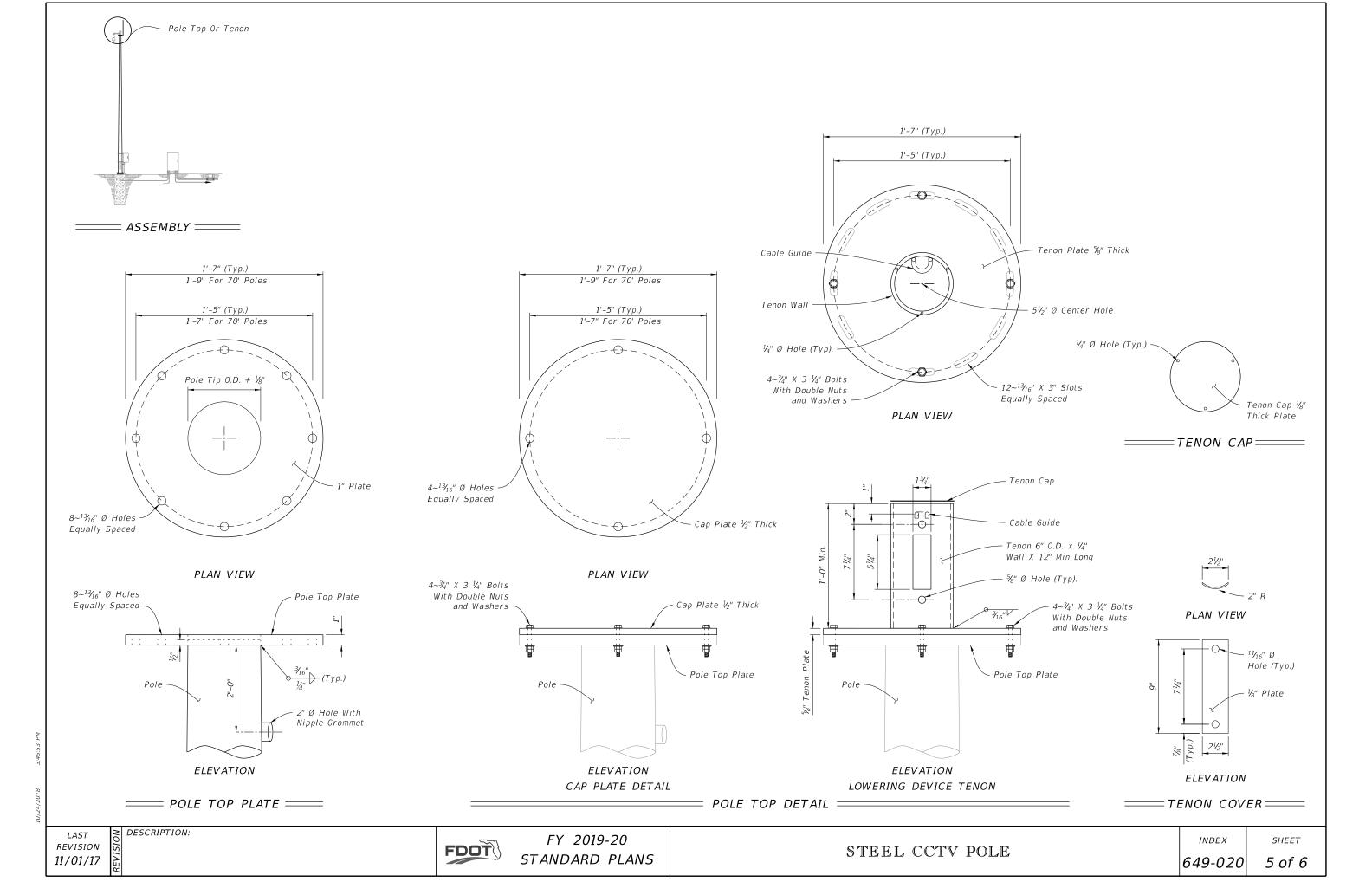
		F	POLE DES	SIGN T	ABLE		
Pole Overall	S	ection 1 (To	p)	Se	ction 2 (Botto	om)	Joint
Height (ft)	Length	Wall Thickness (in.)	Base Diameter (in.)	Length	Wall Thickness (in.)	Base Diameter (in.)	Minimum Splice Length (in.)
50				50'-0"	0.25	17	
30	25'-0"	0.25	14	28'-0"	0.25	17	27
55	30'-0"	0.25	15	28'-0"	0.3125	18	30
60	35'-0"	0.25	18	29'-0"	0.3125	21	33
65	33'-0"	0.25	19	36'-0"	0.3125	23	33
70	38'-0"	0.25	22	36'-0"	0.3125	26	39

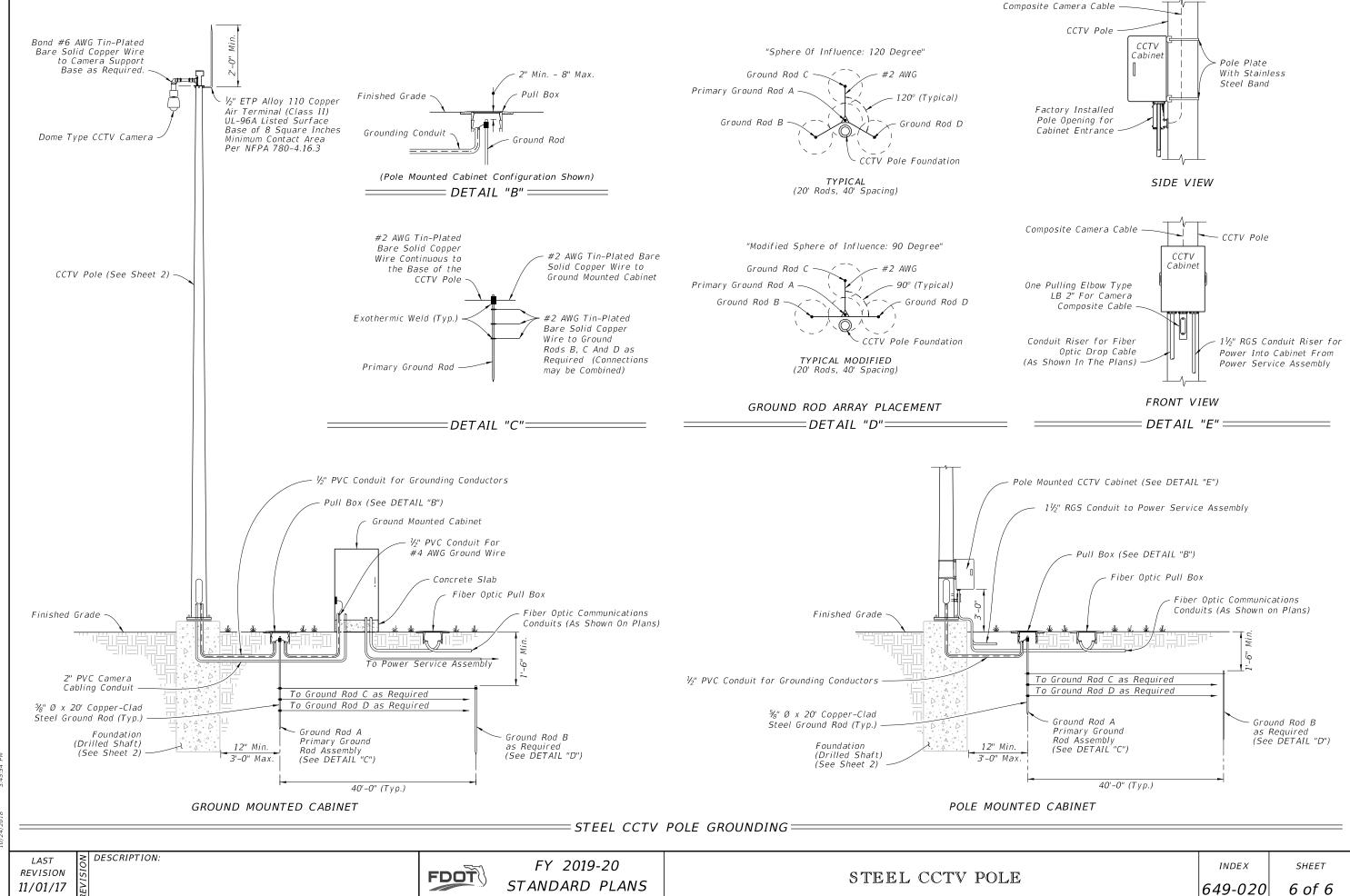


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		,	ARM A	ND BA	SE PL	4 <i>TE</i>				
Arm ID Axx-ArmLength	Total		Arm		Arn	n Extens	sion	Е	Base Pla	te
S-SingleArm D-DoubleArm H-HeavyDuty	Arm Length (ft)	FA/SA (ft)	FC/SC (in)	FD/SD (in)	FE/SE (ft)	FG/SG (in)	FH/SH (in)	HT (in)	FJ/SJ (in)	FK/SK (in)
A30/S		30	11	0.250				22	25	
A30/S/H	30	30	12	0.250				22	23	3
A30/D	] 30	30	11	0.250				30	36	ا ا
A30/D/H		30	12	0.250				30	30	
A40/S		40	13	0.250				22	27	
A40/S/H	40	40	14	0.250				22	2/	3
A40/D	] 40	40	13	0.250				30	36	ا ا
A40/D/H		40	14	0.250				30	30	
A50/S		32.5	12	0.250	20.5	14		22	29	
A50/S/H	50	32.5	13	0.250	20.5	15	0.313	22	29	3
A50/D	] 30	32.5	12	0.250	20.5	14		30	36	] 3
A50/D/H		32.5	13	0.250	20.5	15		30	30	
A60/S		35.5	12	0.250	27.5	15				
A60/S/H	60	35.5	13	0.250	27.5	16	0.375	30	36	.3
A60/D	] 00	35.5	12	0.250	27.5	15	0.373	30	30	3
A60/D/H		35.5	13	0.250	27.5	16				
A70/S		38	13	0.250	35	17				
A70/S/H	70	38	14	0.250	35	18	0.375	30	36	3
A70/D	] //	38	13	0.250	35	17	0.373	30	30	3
A70/D/H		38	14	0.250	35	18				
A78/S		39	13	0.250	42	18				
A78/S/H	78	39	15	0.250	42	20	0.375	30	36	.3
A78/D	] ′°	39	13	0.250	42	18	0.5/5	30	30	د
A78/D/H		39	15	0.250	42	20				

						POLE,	BASE	PLATE	AND	ARM C	ONNEC	TION									
Pole ID Px-PoleNo		Upr	ight			В	ase Pla	te					Arm-Up	right Co	nnection						
S-SingleArm D-DoubleArm L-Luminaire	UA (ft)	UD (in)	UE (in)	UG (ft)	No. Bolts	BA (in)	BB (in)	BC (in)	BF (in)	HT (in)	FJ/SJ (in)	FL/SL (in)	FN/SN (in)	F0/S0 (in)	FP/SP (in)	FR/SR (in)	FS/SS (in)	FT/ST (in)			
P1/S	25									22	25			14		2	8.5				
P1/S/L	39	16	0.375	37.5	6	32	2.5	2	40		23	0.75	0.438	14	1.25		0.5	0.438			
P1/D	25	10	0.575			32	2.5	_	40	30	36	0.75	0.430	23	1.23	2.75	12.5	0.430			
P1/D/L	39			37.5						30	30			23		2.75	12.5				
P2/S	25									22	27			15		2	8.5				
P2/S/L	39	18	0.375	37.5	6	34	2.5	2	40			0.75	0.438	13	1.25		0.5	0.438			
P2/D	25	1	0.575			34	2.5	_	70	30	36	0.75	0.430	23	1.23	2.75	12.5	0.430			
P2/D/L	39			37.5						30				23		2.73	12.3				
P3/S	25									22	29			16		2	8.5				
P3/S/L	39	20	0.375	37.5	6	36	2.5	2	40			0.75	0.438		1.25	_	0,5	0.438			
P3/D	25							_		30	36	""		2.3		2.75	12.5				
P3/D/L	39			37.5																	
P4/S	25													17							
P4/S/L	39	22	0.375	37.5	8	38	2.5	2	40	30	36	0.75	0.438		1.25	2.5	12.5	0.438			
P4/D	25													23							
P4/D/L	39			37.5																	
P5/S	25													18							
P5/S/L	39	24	0.375	37.5	8	40	2.5	2	40	30	36	0.75	0.5		1.25	2.5	12.5	0.5			
P5/D	25													23							
P5/D/L	39			37.5																	
P6/S	25													18							
P6/S/L	39	24	0.5	37.5	8	42	2.5	2.25	45	30	36	0.75	0.625		1.5	2.5	12	0.625			
P6/D	25													23							
P6/D/L	39			37.5																	
P7/S	25													19							
P7/S/L		26	26	39 26	26	6 0.5	37.5	7.5	44	2.5	2.25	45	5 30	0 36	0.75	0.625		1.5	2.5	12	0.625
P7/D	25			0.5				2.5							23						
P7/D/L	39			37.5																	

### NOTE:

≥ DESCRIPTION:

1. Work this Index with Index 649-031.

		DR	ILLED	SHAF	Τ			
Drilled Shaft ID	DA (ft)	DB (ft)	RA	RB	RC	RD (in)	RE	RF (in)
DS/12/4.0	12	4.0	11	14	8	12		
DS/12/4.5	12	4.5	11	16	8	12		
DS/14/4.5	14	4.5	11	16	10	8		
DS/14/5.0	14	5.0	11	18	10	8		
DS/16/4.5	16	4.5	11	16	10	8		
DS/16/5.0	16	5.0	11	18	10	8		
DS/18/5.0	18	5.0	11	18	10	8		
DS/20/5.0	20	5.0	11	18	10	6	10	9
DS/25/5.0	25	5.0	11	18	10	6	10	9

			LUM	INAIF	RE AN	D COI	VNECT	TION			
LA (ft)	LB (ft)	LC (in)	LD (in)	LE	LF (ft)	LG (in)	LH (in)	LJ (in)	LK (in)	LL (deg)	UG (ft)
40	10	3	0.125	0.5	8	0.5	0.75	0.25	0.25	0	37.5

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3. Details for Signal and Sign locations, Signal Head attachment, Sign attachment, Pedestrian Head attachment, and Foundation Conduit are not shown for simplicity.

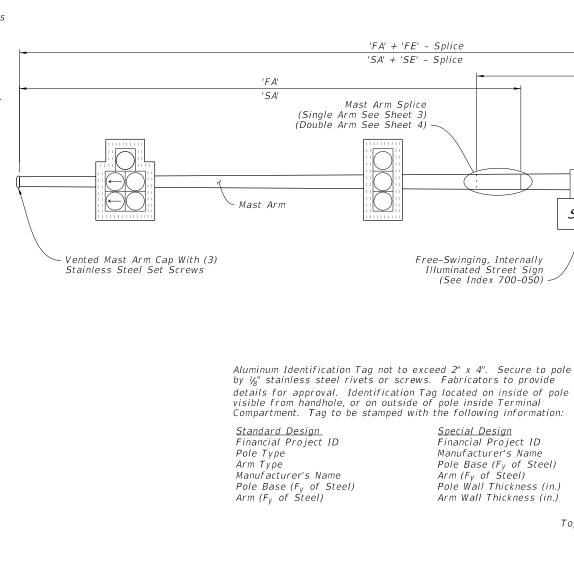
- 4. Materials:
  - A. Poles, Mast Arms and Backing Rings:
    - a. Less than ¾₁₆": ASTM A1011 Grade 50, 55, 60 or 65
    - b. Greater than or equal to  $\frac{3}{16}$ ": ASTM A572 Grade 50, 55, 60 or 65
    - c. ASTM A595 Grade A (55 ksi yield) or Grade B (60 ksi yield)
  - B. Steel Plates: ASTM A36
  - C. Weld Metal: E70XX
  - D. Bolts, Nuts and Washers:
    - a. High Strength Hex Head Bolts: ASTM F3125, Grade A325, Type 1
    - b. Nuts: ASTM A563 DH Heavy-Hex
    - c. Washers: ASTM F436 Type 1, one under turned element
  - E. Anchor Bolts, Nuts and Washers:
    - a. Anchor Bolts: ASTM F1554 Grade 55
    - b. Nuts: ASTM A563 Grade A Heavy-Hex (5 per anchor bolt)
  - c. Plate Washers: ASTM A36 (2 per bolt)
  - F. Threaded Bars/Studs: ASTM A36 or ASTM A307
  - G. Handhole Frame: ASTM A709 or ASTM A36, Grade 36 H. Handhole Cover: ASTM A1011 Grade 50, 55, 60 or 65
  - I. Aluminum Pole Caps and Nut Covers: ASTM B26 (319-F)
  - J. Stainless Steel Screws: AISI Type 316
  - K. Concrete: Class IV (Drilled Shaft) for all environmental classifications.
  - L. Reinforcing Steel: Specification 415

#### 5. <u>Fabrication</u>:

- A. Welding:
- a. Specification Section 460-6.4 and
- b. AASHTO LRFD Specification for Structural Supports for Highway Signs, Luminaires, and Traffic Signals Section 14.4.4
- B. Poles and Mast Arms:
- a. Round or 12-sided (Min.)
- b. Taper pole diameter at 0.14 inches per foot
- c. Upright poles must be a single section. For arms and upright poles, circumferential welds and laminated sections are not permitted.
- d. Arms may be either one or two sections. See Sheet 4 for telescopic splice detail
- e. Fabricate longitudinal seam welds with 60 percent minimum penetration or fusion welds except:
  - 1. Use a full-penetration groove weld within 6 inches of the circumferential tube-to-plate connection.
  - 2. Use full-penetration groove welds on the female end section of telescopic (i.e., slip type) field splices for a minimum length of one and one-half times the inside diameter of the female section plus 6 inches.
- f. Locate longitudinal seams weld along the:
  - 1. Lower quadrant of the arms.
  - 2. Same side of the pole as the arm connections
- g. Face handhole perpendicular from arm on single arm poles, perpendicular from the first arm of double arms poles facing away from traffic or see special instructions on the Mast Arm Tabulation Sheet.
- h. Provide a 'J' or 'C' hook at the top of the pole for signal wiring support (See Sheet 6)
- i. First and Second arm camber angle =  $2^{\circ}$
- j. Bolt holes diameters as follows:
  - 1. Bolts (except Anchor bolts): Bolt diameter plus 1/16" prior to galvanizing.
  - 2. Anchor Bolts: Bolt diameter plus ½" (Max.).
- 6. Coatings:
  - A. All Nuts, Bolts, Washers and Threaded Bars/Studs: ASTM F2329
  - B. All other steel items including plate washers ASTM A123
- - A. Foundation: Specification 455 Drilled Shaft, except that payment is included in the cost of the Mast Arm.
  - B. Install Pole vertically.

DESCRIPTION:

- C. Place structural grout pad with drain between top of foundation and bottom of baseplate in accordance with Specification 649-7.
- D. Attach Sign Panels and Signals centered on the elevation of the Mast Arm.
- E. Wire Access holes are 11/2" or less in diameter



Financial Project ID
Manufacturer's Name
Pole Base $(F_y \text{ of Steel})$
$Arm (F_V of Steel)$
Pole Wall Thickness (in.)
Arm Wall Thickness (in.)

Special Design

Free-Swinging, Internally Illuminated Street Sign

(See Index 700-050)

'FA' + 'FE' - Splice

'SA' + 'SE' - Splice

Mast Arm Splice

(Single Arm See Sheet 3) (Double Arm See Sheet 4)

Handhole (See Sheet 6) Base Plate Connection (See Sheet 2) Bottom Top of Finished Grade Of Plate O" With Sidewalk 6" Otherwise Signal Conduit 1~2" Conduit Per Assembly (For No. & Size 1~1" Additional Conduit in See Signal Plans) Quadrant With Controller Foundation (Drilled Shaft) (See Sheet 2)

Pole

Face Of Arm Base Plate At G Arm -

Pole Connection

0.14 in/ft Taper (Typ.)

Mast Arm

Extension

(Single Arm See Sheet 3)

(Double Arm See Sheet 4)

Provide 1/2" Ø Weep Hole

Located At Bottom Of Arm.

1'-0" From Arm Base Plate.

'FE'

'SE'

Street Name

€ Pole

Pole Top

Mast Arm

Handhole

(See Sheet 6)

(See Sheet 6)

649-030,

Plans) (See

(See

'F0'

'S0'

TABLE OF CONTENTS SHEET SUBJECT Elevation and Notes Foundation and Base Plate Details Single Arm Connection and Splice Details 3 Double Arm Connection and Splice Details Luminaire Arm and Connection Details Handhole and Pole Top Details

= MAST ARM ASSEMBLY ==

Single Arm Shown, Double Arm Similar

(Luminaire Arm Not Shown)

ELEVATION AND NOTES

**REVISION** 11/01/18

**FDOT** 

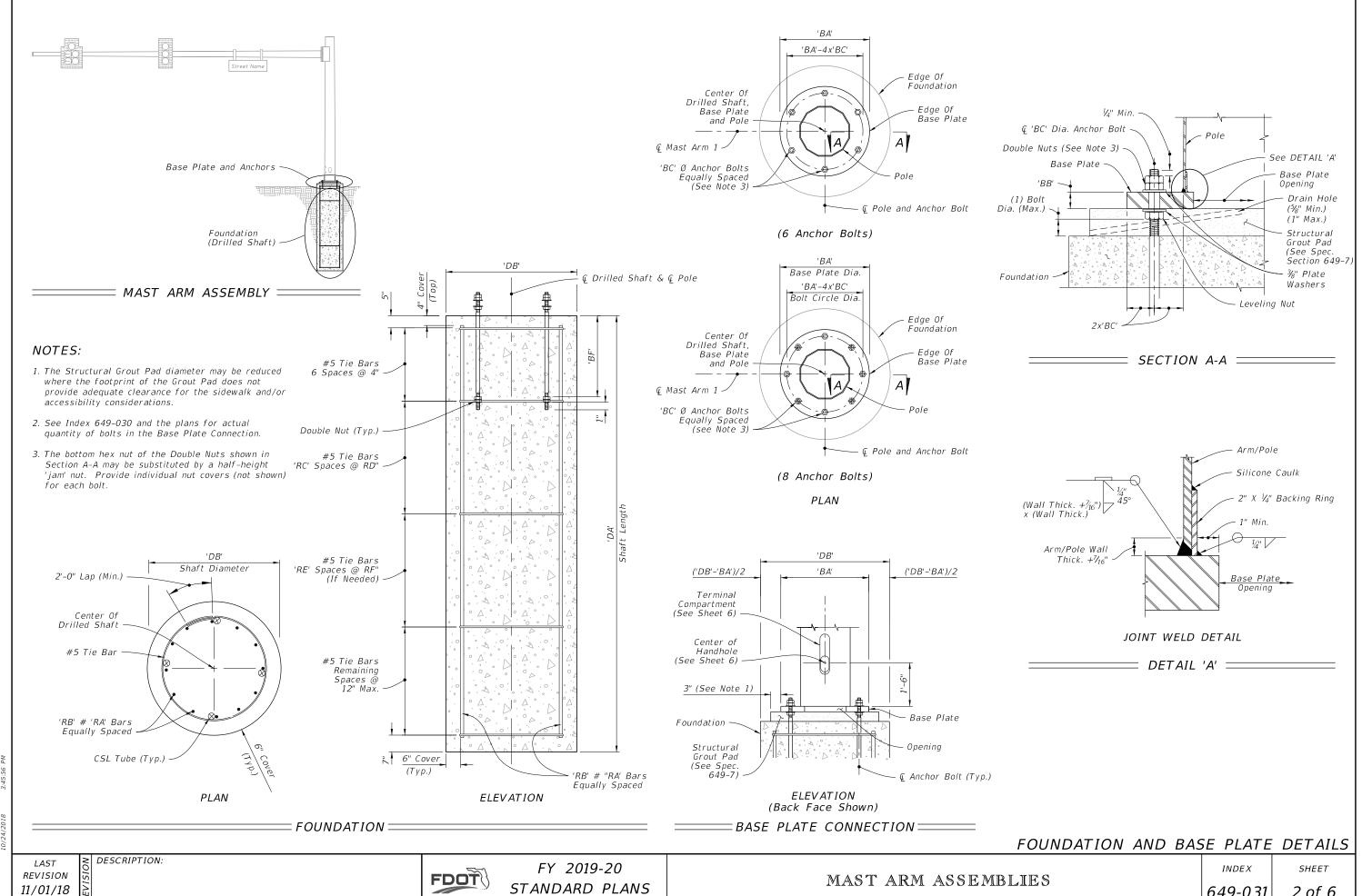
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MAST ARM ASSEMBLIES

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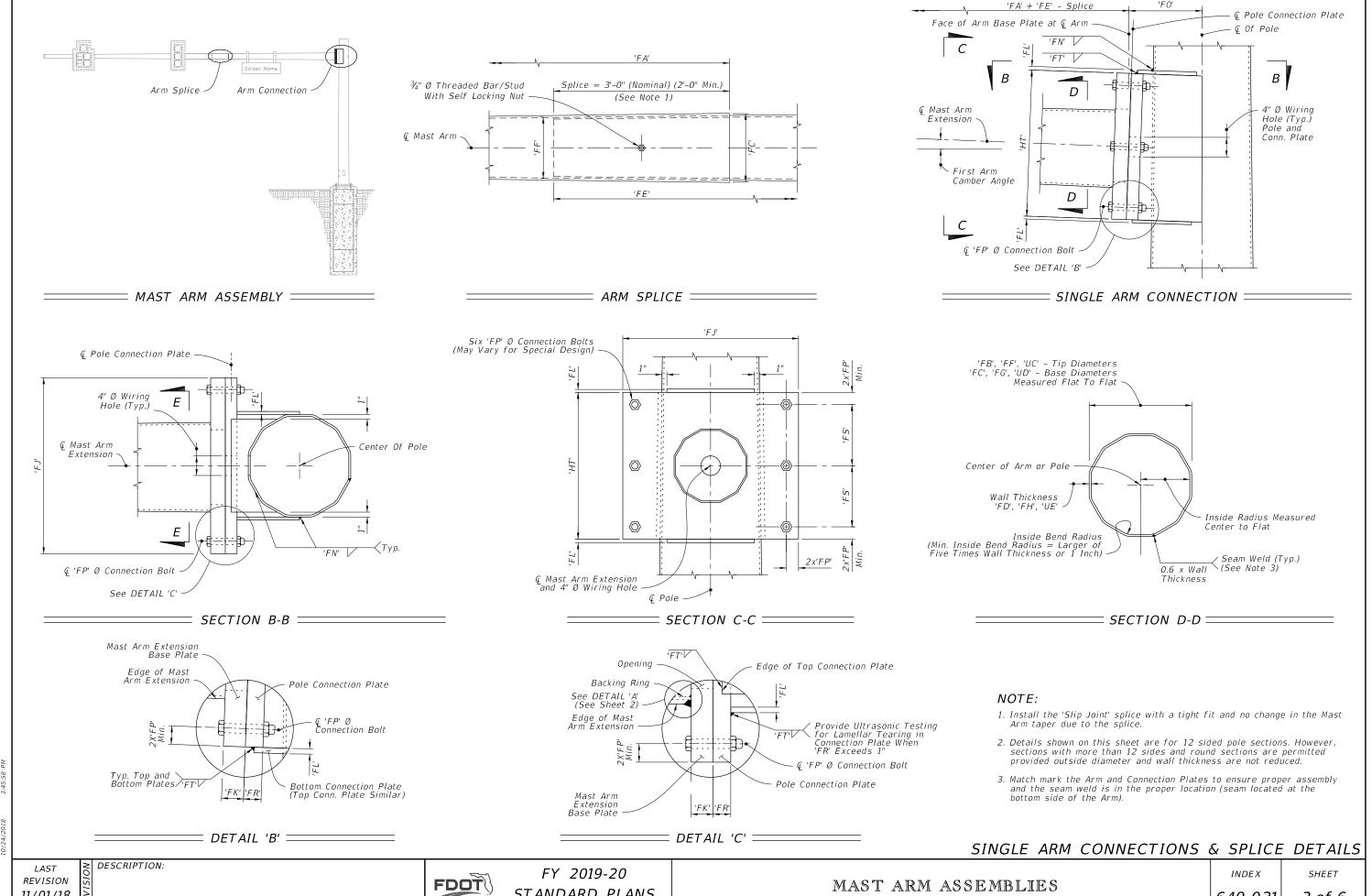
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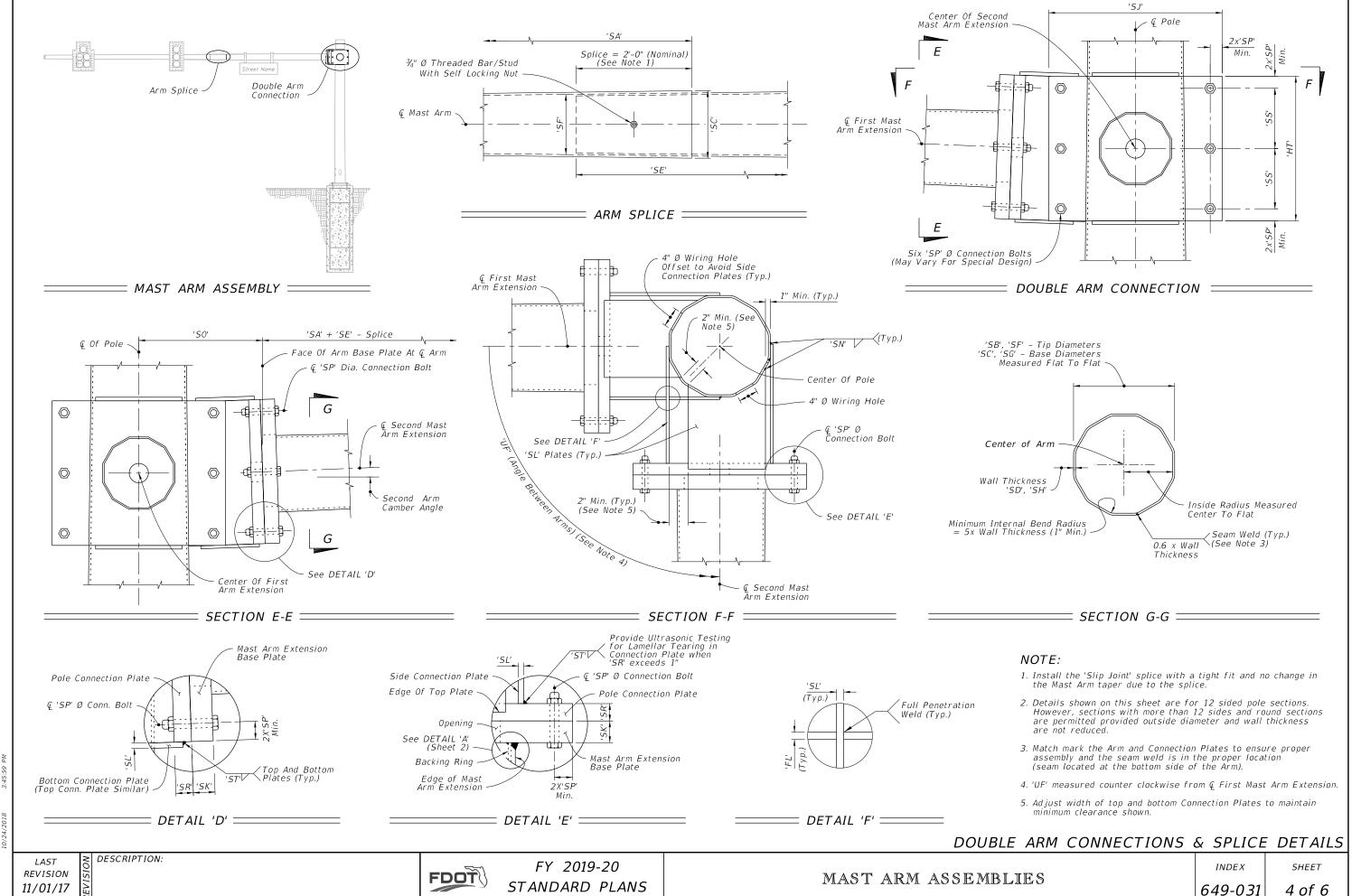
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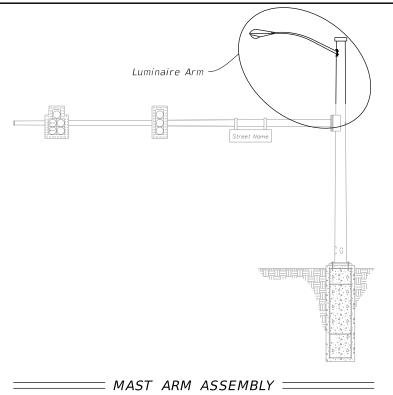
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STANDARD PLANS

649-031

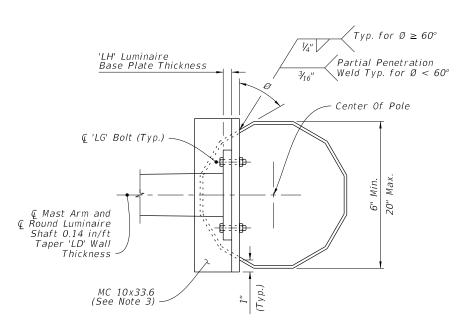
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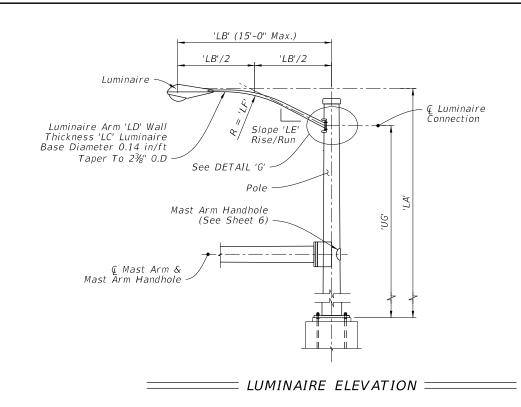


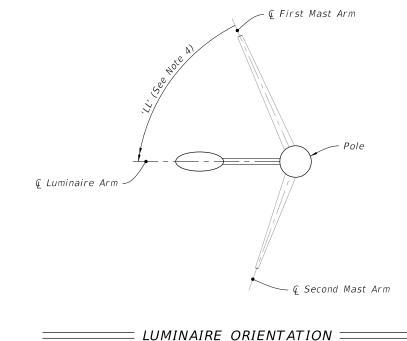
#### NOTES:

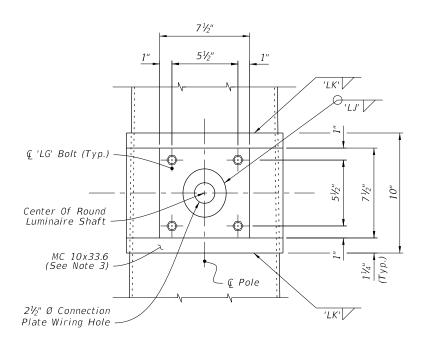
- 1. Luminaire type and luminaire length may be found in the Lighting Plans.
- 2. Align Luminaire Arm with Single Mast Arm or First Arm of Double Mast Arm unless indicated otherwise in the plans.
- 3. The fabricator may substitute a  $\frac{1}{2}$ " thick bent plate with the same flange width, height, and length as the MC 10x33.6 Channel section.
- 4. 'LL' measure counter clockwise from First Mast Arm.

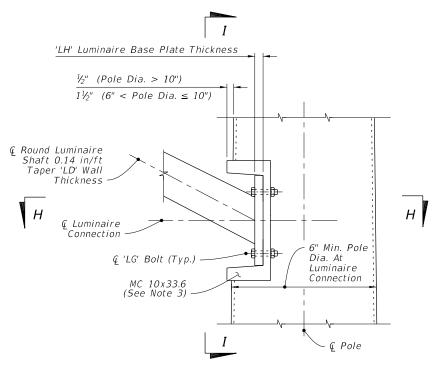


= SECTION H-H =









LUMINAIRE CONNECTION ELEVATION

= DETAIL 'G' =====

LUMINAIRE ARM AND CONNECTION DETAILS

**REVISION** 11/01/17

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FY 2019-20 STANDARD PLANS

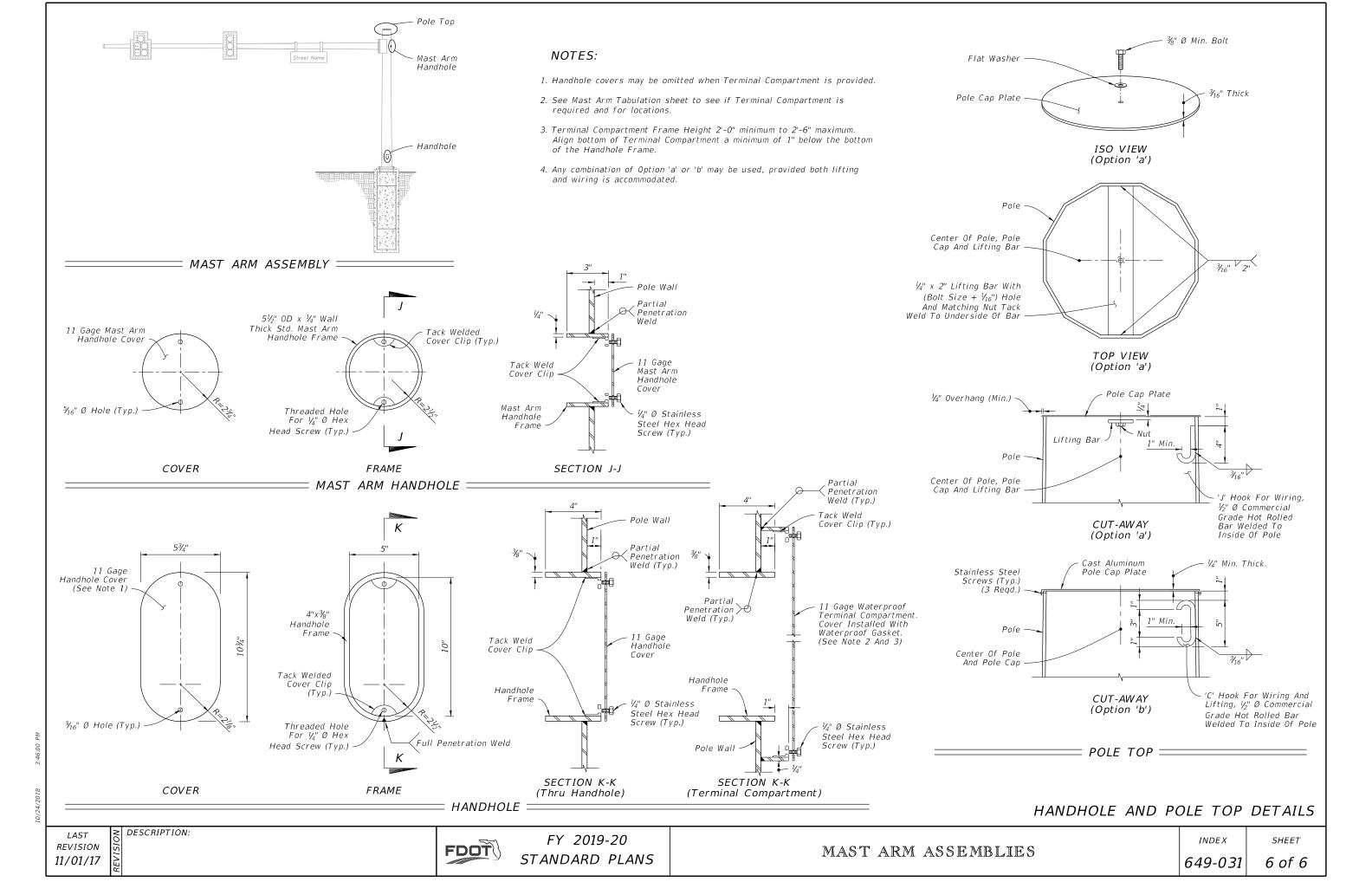
MAST ARM ASSEMBLIES

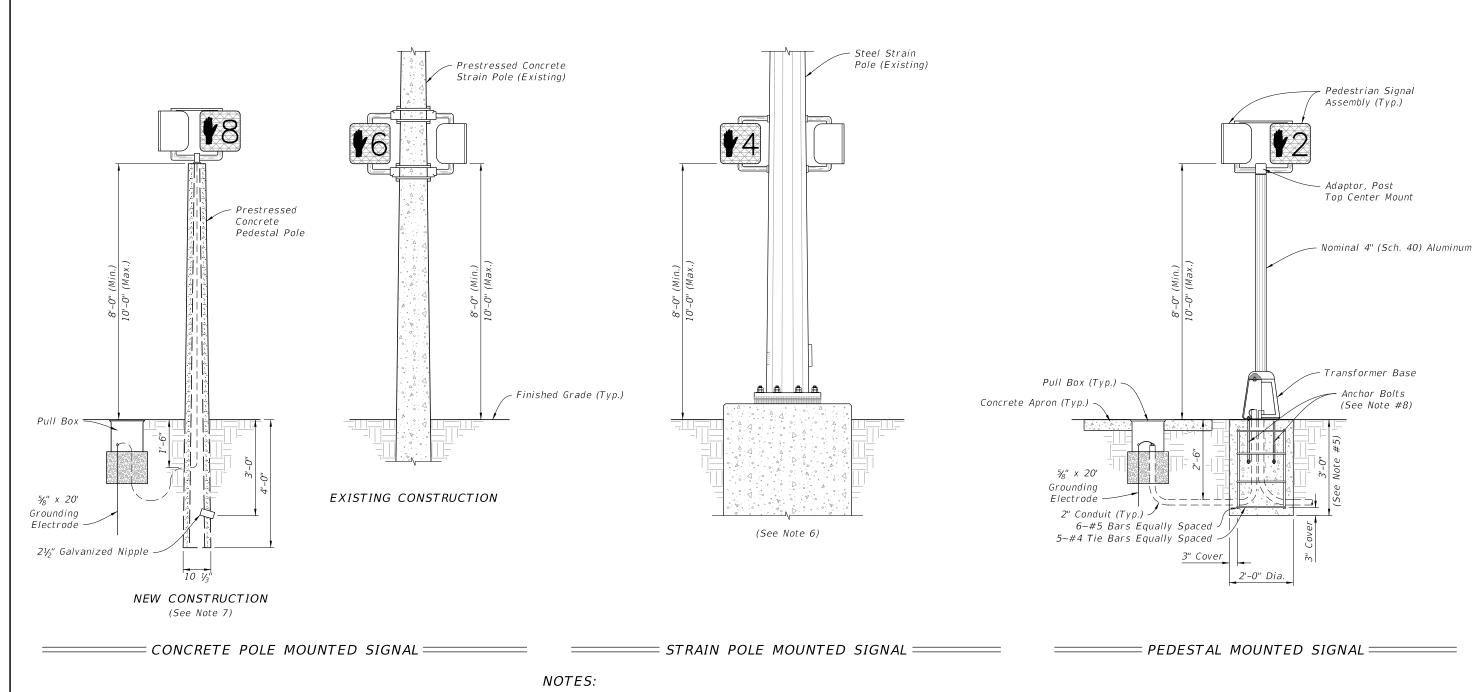
INDEX

SHEET

DESCRIPTION:

= SECTION I-I =





- 1. As an option, pedestrian signals may be installed on concrete poles and pedestals using lead anchors (two bolts same size per hub) in lieu of the stainless steel bands.
- 2. Repair drilled or punched holes in galvanized steel poles or pedestals in accordance with Specifications 562. Install grommets or bushings in each hole.
- 3. Meet grounding requirements of Specifications 620.
- 4. See APL for Department-approved Pedestrian Signal Assemblies and hardware.
- 5. Construct footing with Class I Concrete, footing may be Cast-In-Place (CIP) or Precast.
- 6. For Steel Strain Poles see Index 649-010.
- 7. For Prestressed Concrete Poles see Index 641-010.
- 8. Install 4  $\sim \frac{3}{4}$ " x 18" Anchor Bolts With Double Nuts. (ASTM F1554 Grade 55)
- 9. Meet the requirements of Specifications 646 for aluminum poles and transformer bases.

**REVISION** 11/01/17

DESCRIPTION:

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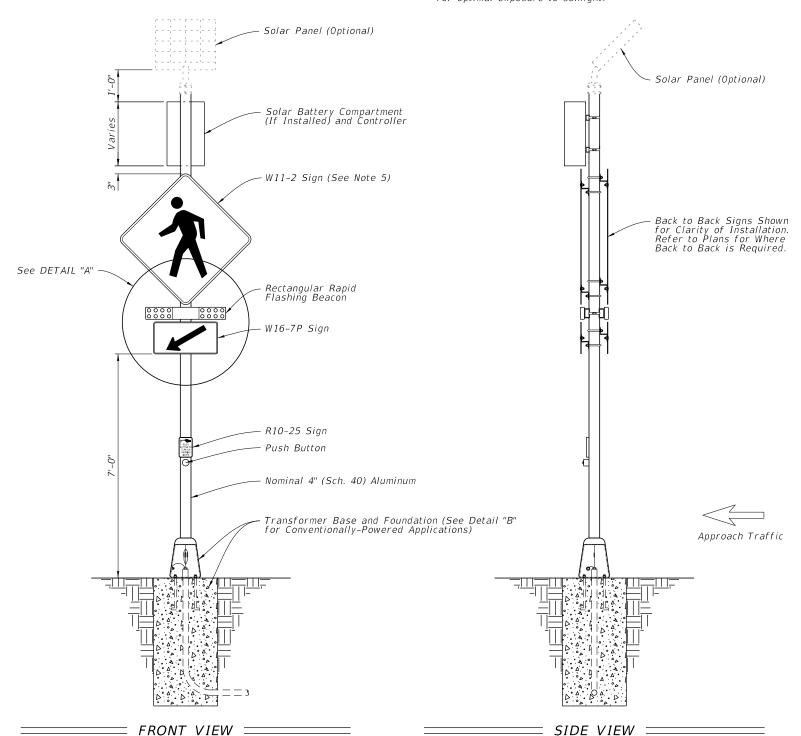
SHEET

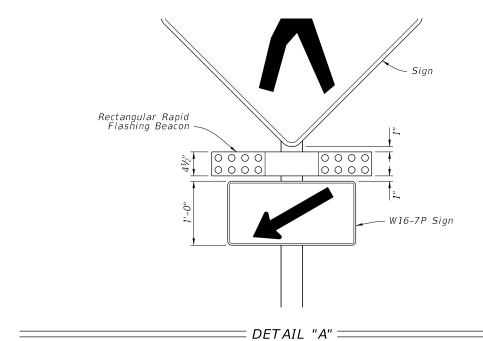
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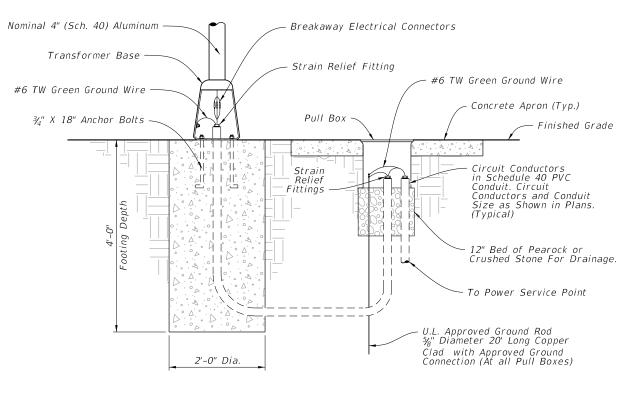
#### **NOTES:**

- 1. A transformer base is required for both conventionally-powered and solar-powered applications (conventional power shown).
- 2. Install the RRFB in pairs, one on either side of approach traffic.
- 3. Install controller on the backside of post from approach traffic.
- 4. Install a 30" X 30" W11-2 sign on two-lane roadways and a 36" X 36" W11-2 sign for multilane roadways.
- 5. Install push button and R10-25 sign in accordance with Index 665-001.
- 6. Engage all threads on the transformer base and post unless the aluminum post is fully seated into base.

- 7. Meet the requirements of Specifications 646 for aluminum poles and transformer bases.
- 8. Install a concrete slab around all pull boxes. The minimum slab dimension is 4'-0" by 4'-0". In urban areas where space is limited slab dimensions may be adjusted as shown in the Plans.
- 9. For assemblies connected to conventional power, provide single pole non-fused watertight breakaway electrical connectors in the frangible transformer base.
- 10. When wire entry holes are drilled in the sign column, use a bushing or rubber grommet to protect conductors.
- 11. For solar-powered applications, orient solar panel to face South for optimal exposure to sunlight.







POLE WIRING AND FOOTING DETAIL

DETAIL "B"

**REVISION** 11/01/18

DESCRIPTION:

**FDOT** 

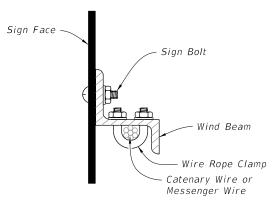
FY 2019-20 STANDARD PLANS

654-001

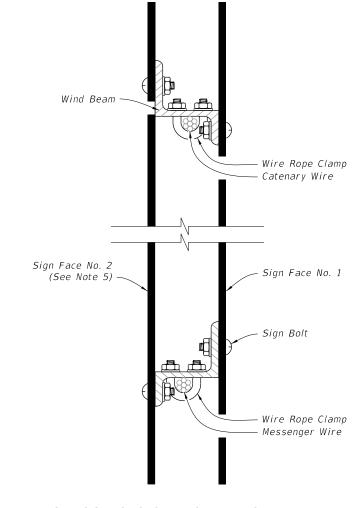
- 1. Materials:
- A. Sign panels, wind beams and associated hardware: See Index 700-020
- B. Sign adjustable hangers, wire rope clamps and associated hardware: See APL
- C. Wire and additional hardware requirements: See Specification 634
- 2. Type B and C Attachments:
- A. Extend wind beams to within 6" of the sign edge.
- B. Number of sign hangers required based on sign width:
- a. Sign width < 4'-0": One
- b. 4'-0" ≤ sign width ≤ 7'-0" : Two
- C. Number of wind beams required based on sign depth:
- a. Sign depth < 3'-6": One
- b. 3'-6" ≤ Sign depth ≤ 7'-0": Two
- 3. Type D Attachments:

Maximum sign width = 3'-0"

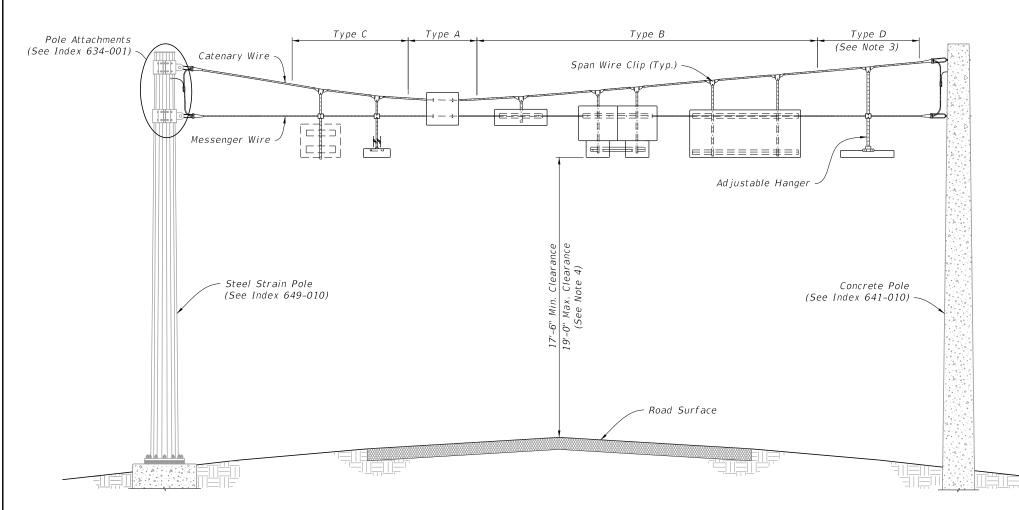
- 4. Align the bottom edges of signs to approximately the same elevation.
- 5. Use a minimum of 2 bolts with a minimum spacing of 2" for overlapped connection of the adjustable hangers.



====SIGN MOUNTING DETAIL====



=OPPOSING SIGN MOUNTING DETAIL====



= TYPICAL INSTALLATIONS FOR SIGN PANEL(S) MOUNTED ON SPAN WIRE =

9:19:09

LAST REVISION 11/01/18

FDOT

FY 2019-20 STANDARD PLANS

SPAN WIRE MOUNTED SIGN DETAILS

1NDEX 659-010

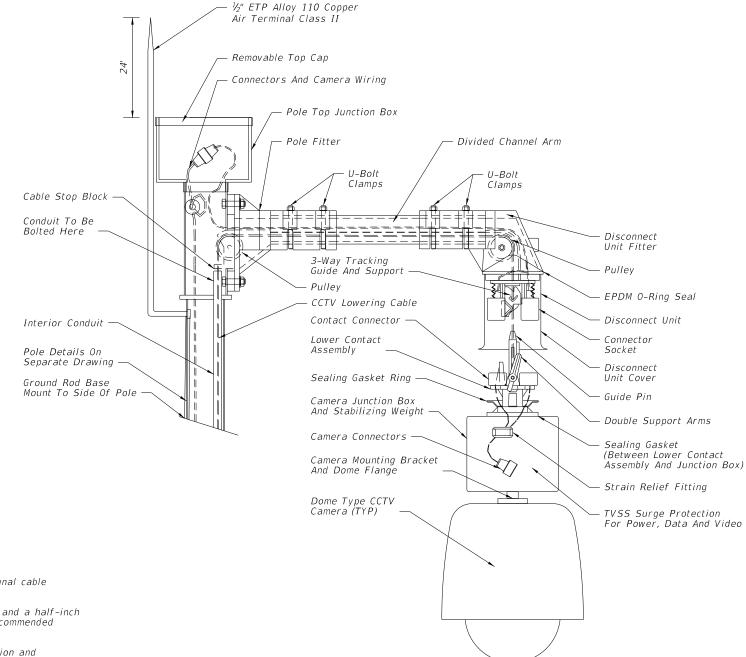
*SHEET* **1 of 1** 





LAST





### **GENERAL NOTES:**

- 1. Lowering device to be shipped ready for pole attachment to include 100 ft. of composite power and signal cable prewired to lowering device at the factory.
- 2. The lowering device manufacturer shall supply both a portable lowering tool with a manual hand crank and a half-inch chuck variable-speed reversible industrial-duty electric drill that matches the winch's manufacturer-recommended revolutions per minute. One lowering tool per every 10 lowering devices is required.
- 3. The lowering device manufacturer shall provide an on-site installation inspection and operator instruction and certification. This ensures the product is assembled correctly and that all necessary persons are trained in the proper, safe operation of the system. Before erecting the first pole the contractor must contact the lowering device supplier and schedule a manufacturer's representative to be on-site.
- 4. Design camera mounting arm and connection to tenon according to FDOT Structures Manual (current edition).
- 5. Camera to be mounted to camera junction box and stabilizing weight via 1½" Standard NPT Pipe Thread.
- 6. Use air terminal extension when the pole top junction box is wider than top of pole.
- 7. The stainless steel device lowering cable shall be installed inside the pole within a 1 1/4" diameter PVC conduit.
- 8. All communication and power cables must be neatly bundled and secured.
- 9. Use a Camera Lowering Device listed on the Approved Product List (APL).
- 10. See Index 641-020 for concrete pole details and Index 649-020 for steel pole details.

CAMERA MOUNTING WITH LOWERING DEVICE

CAMERA LOWERING DEVICE DETAIL

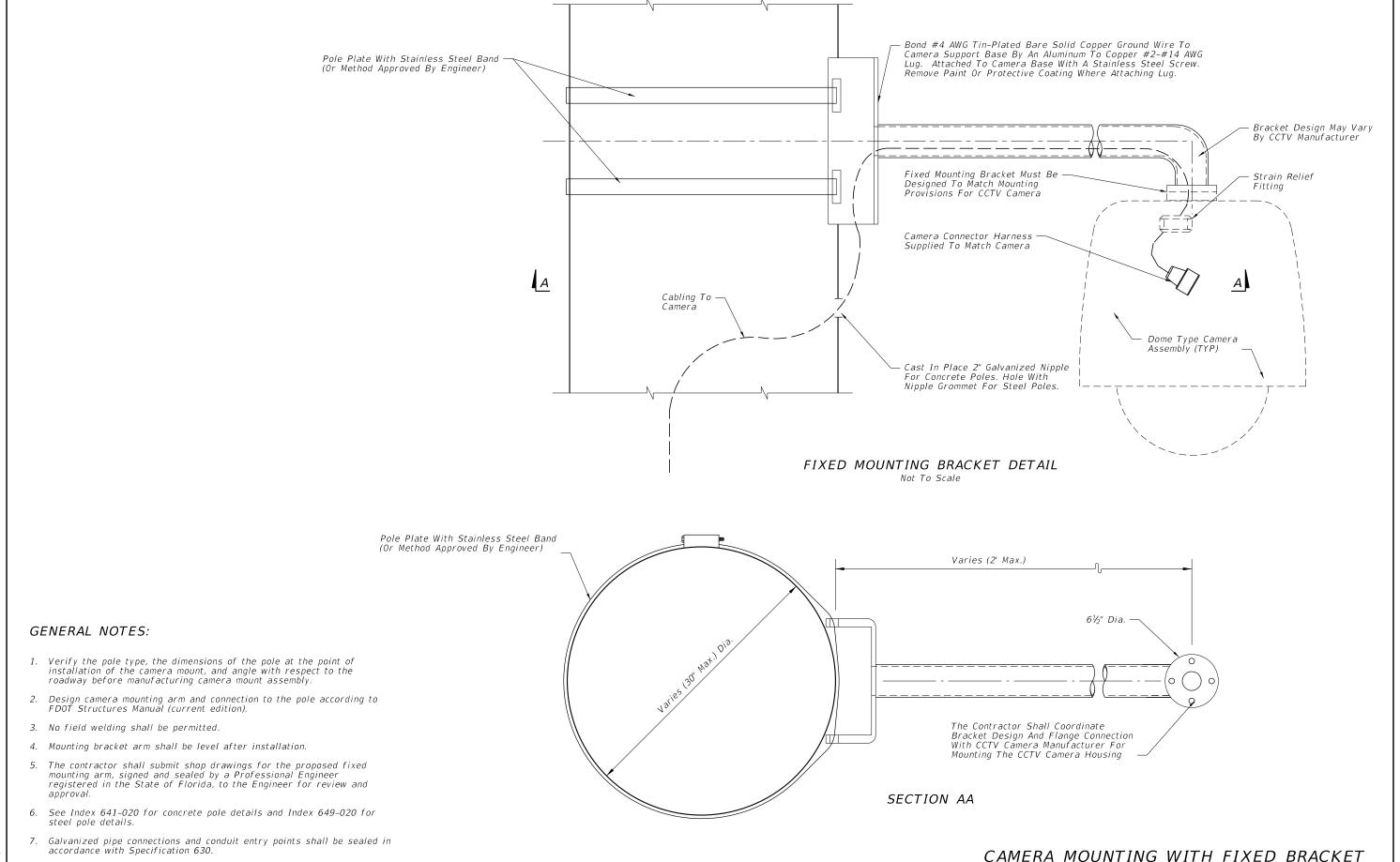
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**REVISION** 

STANDARD PLANS



NO 50.9V.5

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**REVISION** 

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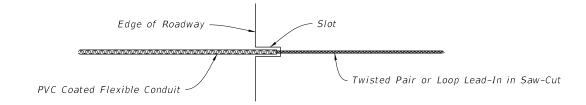
CAMERA MOUNTING DETAILS

INDEX 659-020

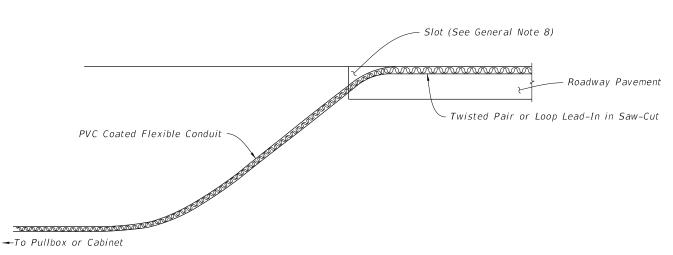
*SHEET* 2 of 2

### GENERAL NOTES:

- 1. If the loop lead-in is 75' or less from the edge of the loop detector to controller cabinet, continue the twisted pair to the cabinet. If the loop lead-in is greater than 75' continue the twisted pair an Intermediate Pullbox, splice to shielded lead-in wire and continue to the controller cabinet.
- 2. Provide sufficient saw-cut width to allow unforced placement of loop wires or lead-in cables into the saw-cut. Except across expansion joints, saw-cut to a standard depth of 3", but no more than 4" below the top of the final surface.
- 3. On resurfacing or new roadway construction projects, install the loop wires and lead-in cables in the asphalt structural course prior to the placement of the asphalt friction course. Place the loop wires and lead-in cables in a saw cut in the structural course.
- 4. Use nonmetallic hold down material to secure loop wires and lead-ins to the bottom of saw-cuts. Place the hold down material approximately 12" intervals around loops and 24" intervals on lead-ins.
- 5. The minimum distance between the twisted pairs of loop lead-in wire is 6" from the loop to 12" from the pavement edge or curb.
- 6. Splice Connections in pull boxes with UL listed, watertight insulated enclosures. Place one enclosure over the end of each conductor and place a third enclosure over the exposed end of the shielded cable. As an alternate, a larger diameter enclosure that will accommodate both the splices of the conductors and the exposed end of the shielded cable may be used.
- 7. Do not disturb more than a 6" x 6" area of asphalt. Restore asphalt as directed by the Engineer.
- 8. Alternative installations may be approved by the State Traffic Operations Engineer.



#### PLAN



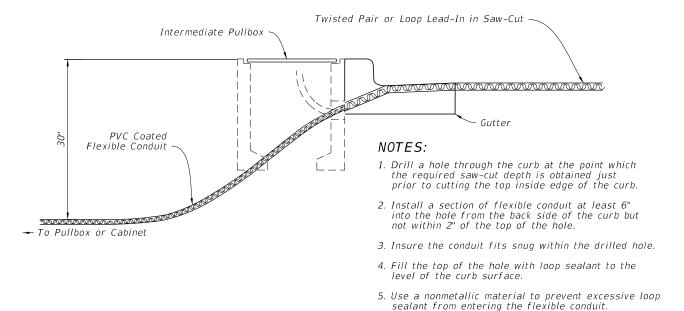
### **NOTES:**

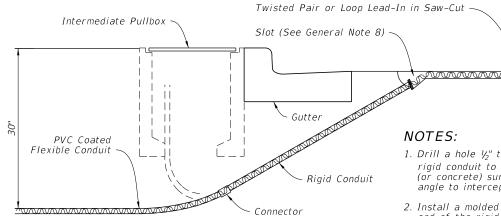
- 1. Cut a slot in the edge of the roadway of sufficient size and depth to snugly place the end of the flexible conduit.
- 2. Install the conduit at least 6" into the roadway pavement and approximately 2" below the top of the roadway surface.
- 3. The departure angle of the conduit from the roadway is between 30° to 45°.

### **ELEVATION**

### = INSTALLATION WITHOUT CURB & GUTTER =

→ To Pullbox or Cabinet





- 1. Drill a hole 1/2" to 1" larger in diameter than the rigid conduit to be used through the roadway asphalt (or concrete) surface and base at an appropriate angle to intercept the trench or pull box hole.
- 2. Install a molded bushing (nonmetallic) on the roadway
- 3. Place the top of the rigid conduit approximately 2" below the roadway surface.
- 4. Fill the hole with loop sealant to the level of the roadway surface.
- 5. Use a nonmetallic material to prevent excessive loop sealant from entering the rigid conduit.

ALTERNATIVE 1 ALTERNATIVE 2

INSTALLATION WITH CURB & GUTTER

## TWISTED PAIR AND LOOP LEAD-IN INSTALLATION

**REVISION** 11/01/18

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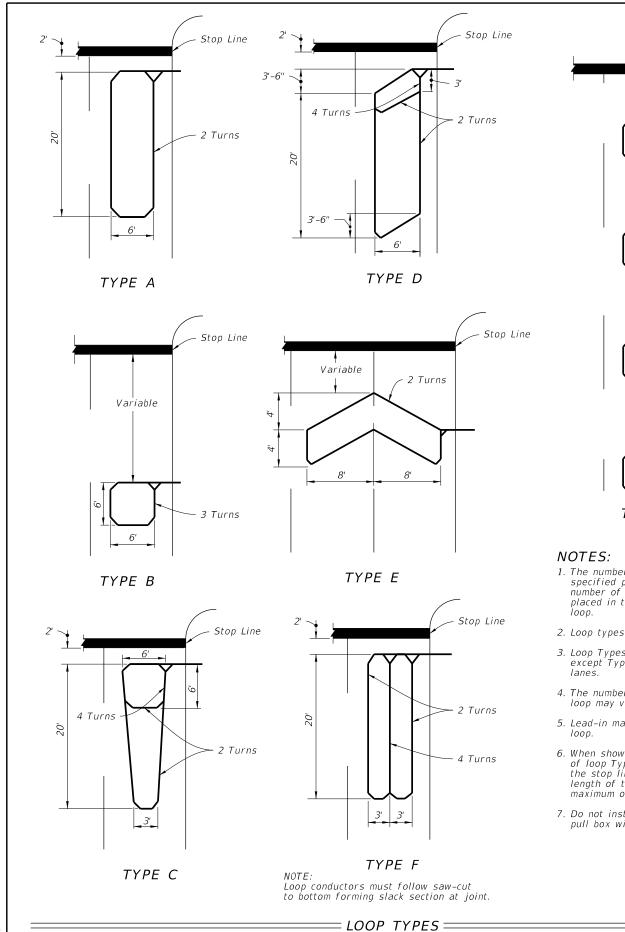
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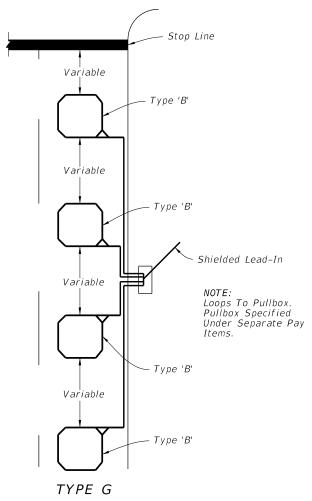
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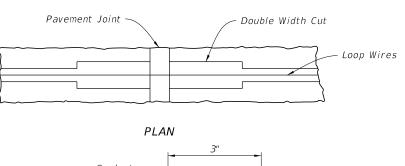
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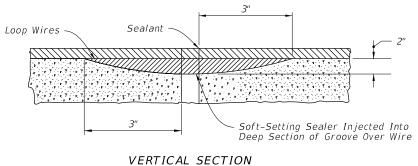
660-001



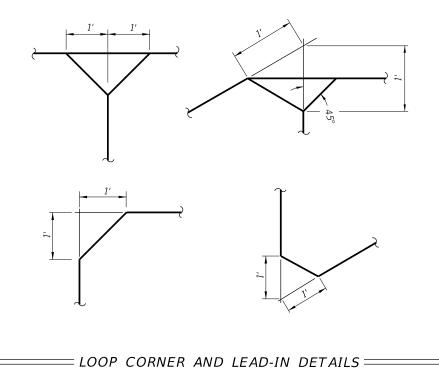


- The number of "Turns" indicated at the specified point on the loop refers to the number of passes of loop wires which are placed in the saw-cut forming the complete loop.
- 2. Loop types or details not drawn to scale.
- 3. Loop Types are centered in a single lane except Type E which is centered on two lanes.
- 4. The number of individual loops in the Type G loop may vary up to a maximum of four (4).
- 5. Lead-in may be connected to either end of loop.
- 6. When shown in the Plans, the leading edge of loop Types A, C, D, & F may extend past the stop line a maximum of 10' and the length of these loops may be extended to a maximum of 60'.
- 7. Do not install loop lead-in wires in the same pull box with signal power cable.





CONCRETE PAVEMENT EXPANSION JOINTS



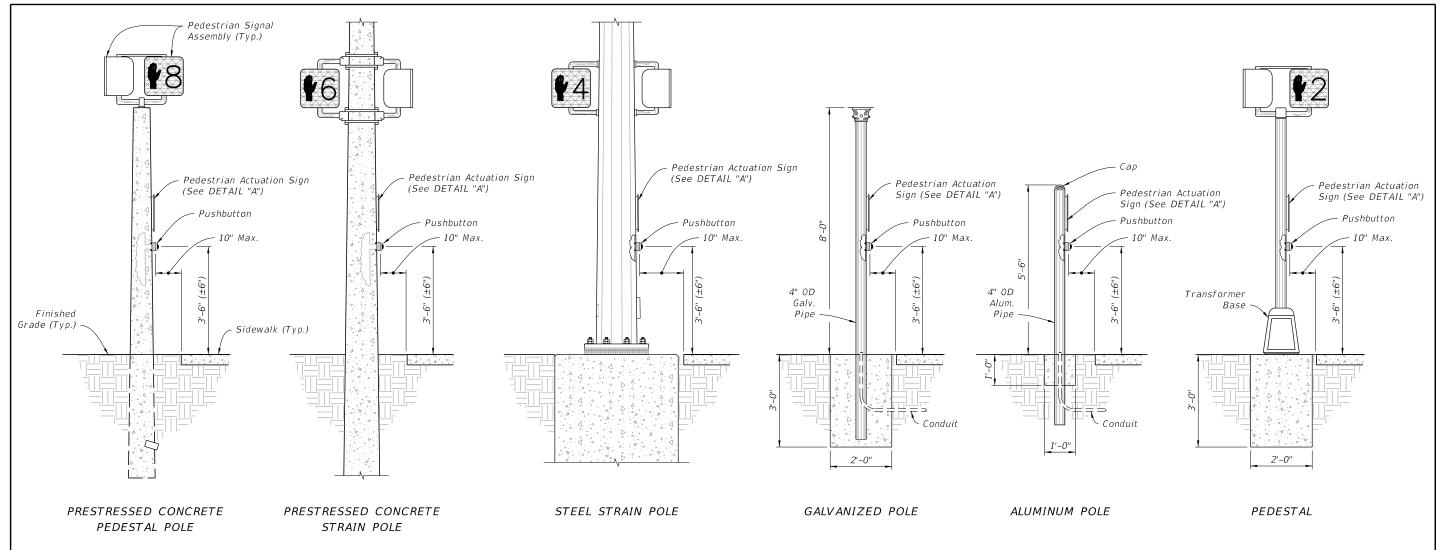
IOOD TYPES EVRANCION IOINTS AND DETAIL

LAST REVISION 11/01/18

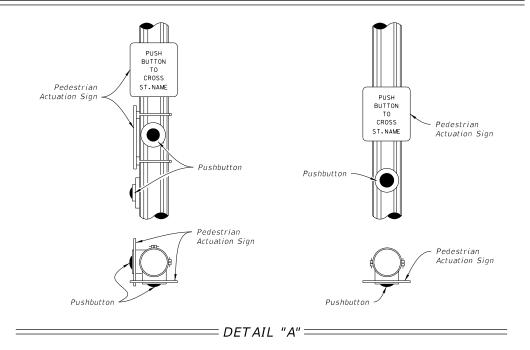
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS LOOP TYPES, EXPANSION JOINTS, AND DETAILS







- 1. Mount Signs above the detectors. See Index 700-102 for sign details.
- 2. Install Pushbuttons and Pedestrian Actuation Signs with faces parallel to the crossing direction.
- 3. Mount pushbuttons and Signs in accordance with Specification 665.
- 4. Install all grounding per Specification 620.
- 5. Pushbutton mounting height shown above is taken at the center of the actuation switch.

≥ DESCRIPTION: LAST REVISION 11/01/18

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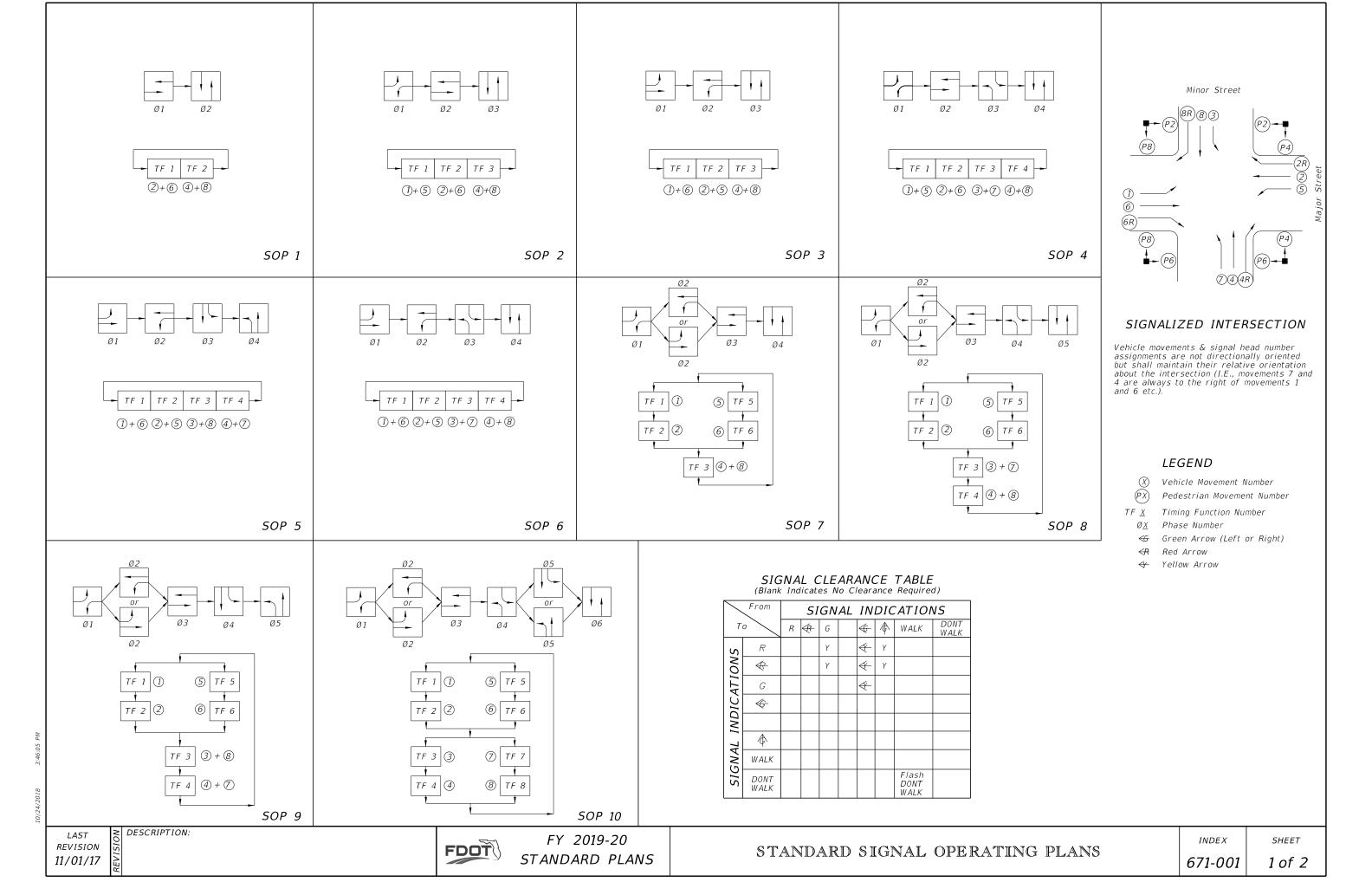
FY 2019-20 STANDARD PLANS PEDESTRIAN DETECTOR ASSEMBLY INSTALLATION DETAILS

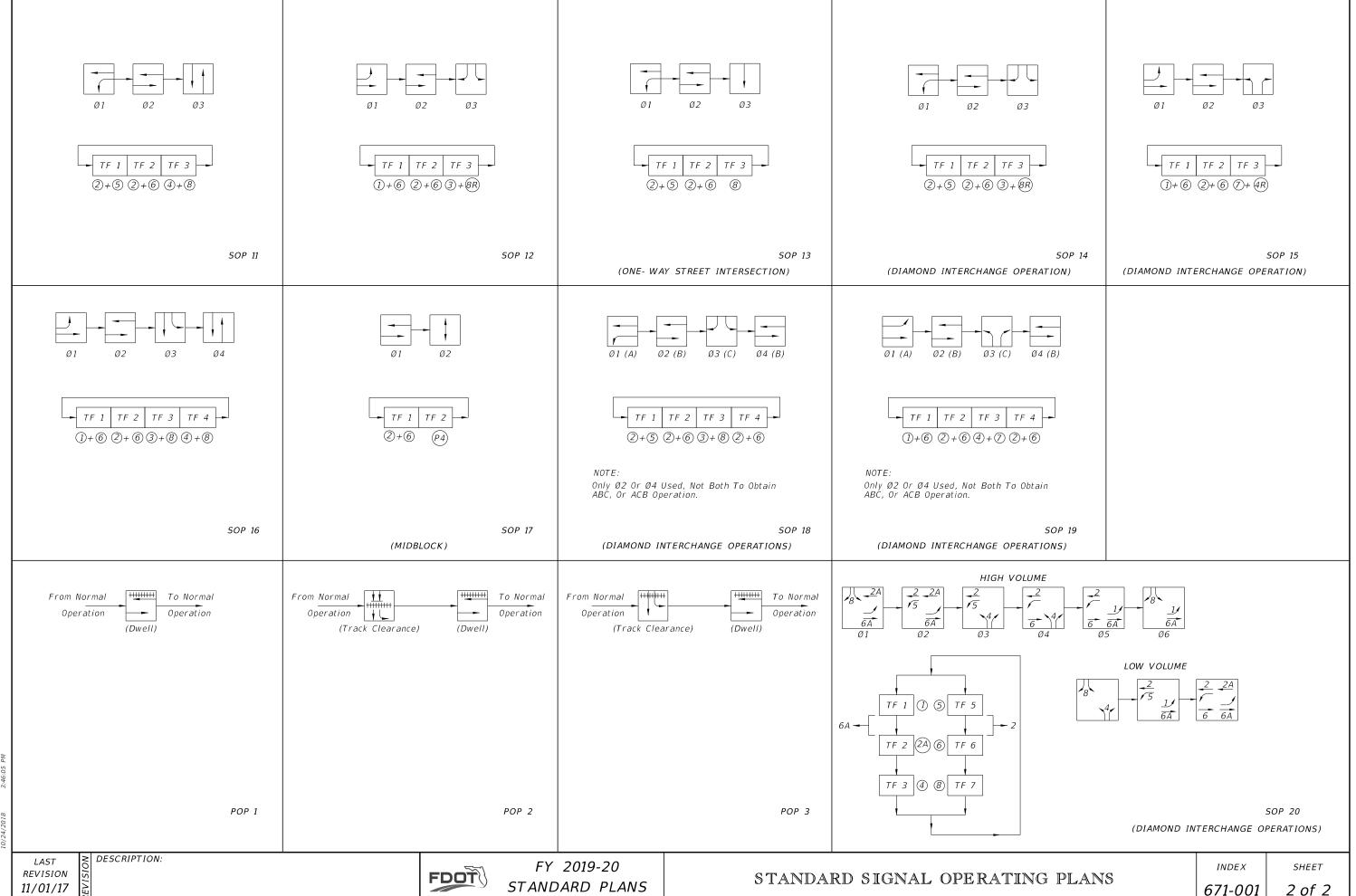
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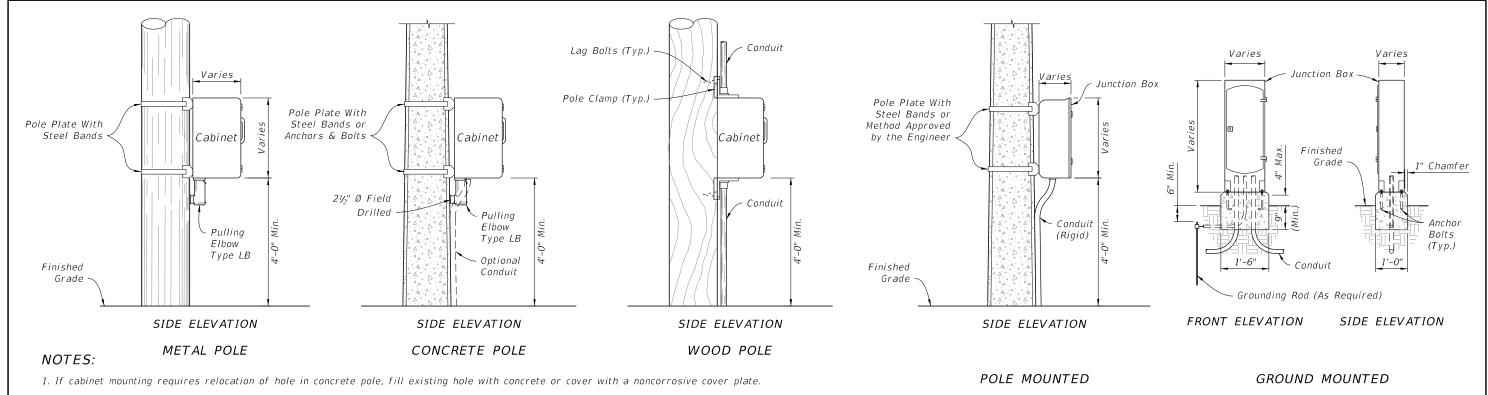
SHEET

665-001

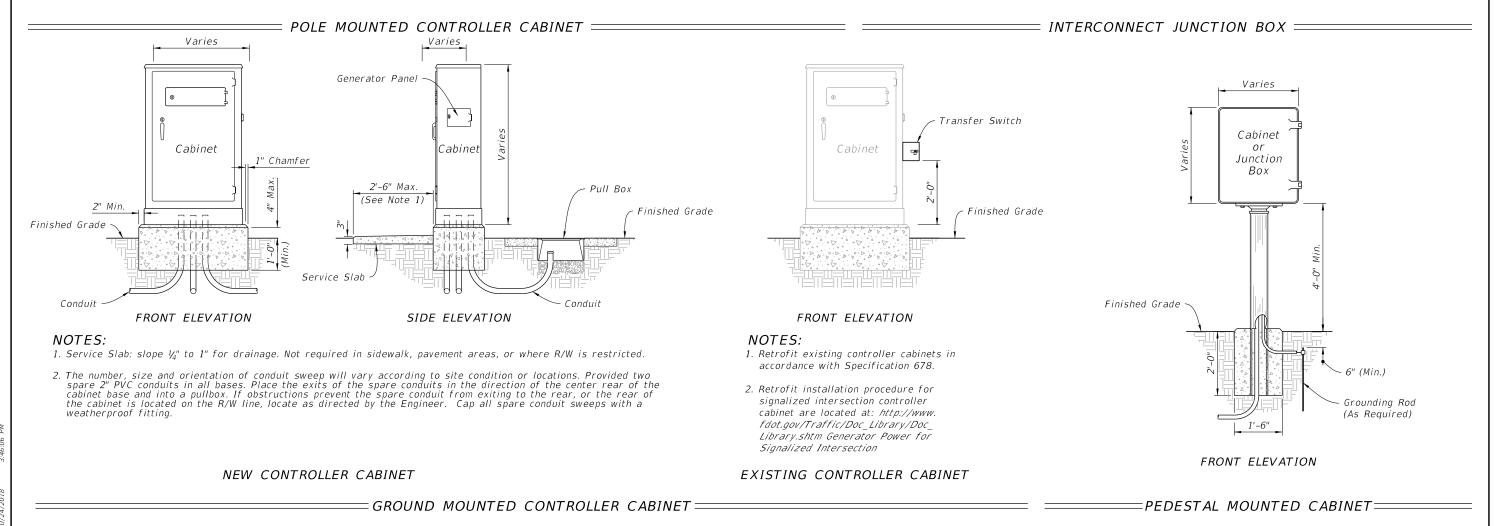
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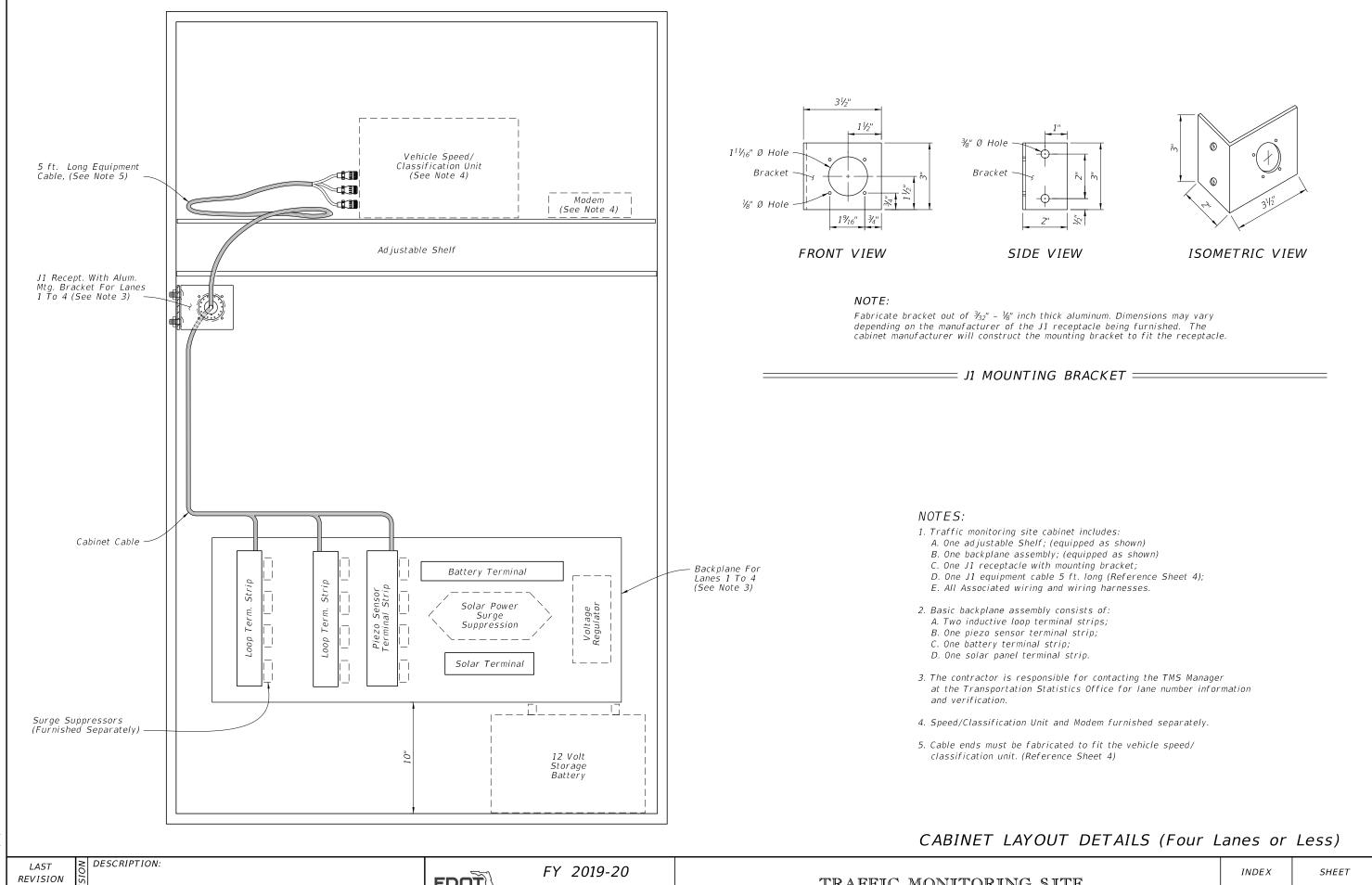
2. Liquidtight flexible conduit is approved for use from the electrical disconnect to the cabinet when both are installed on the same pole.



**REVISION** 

11/01/18

DESCRIPTION:



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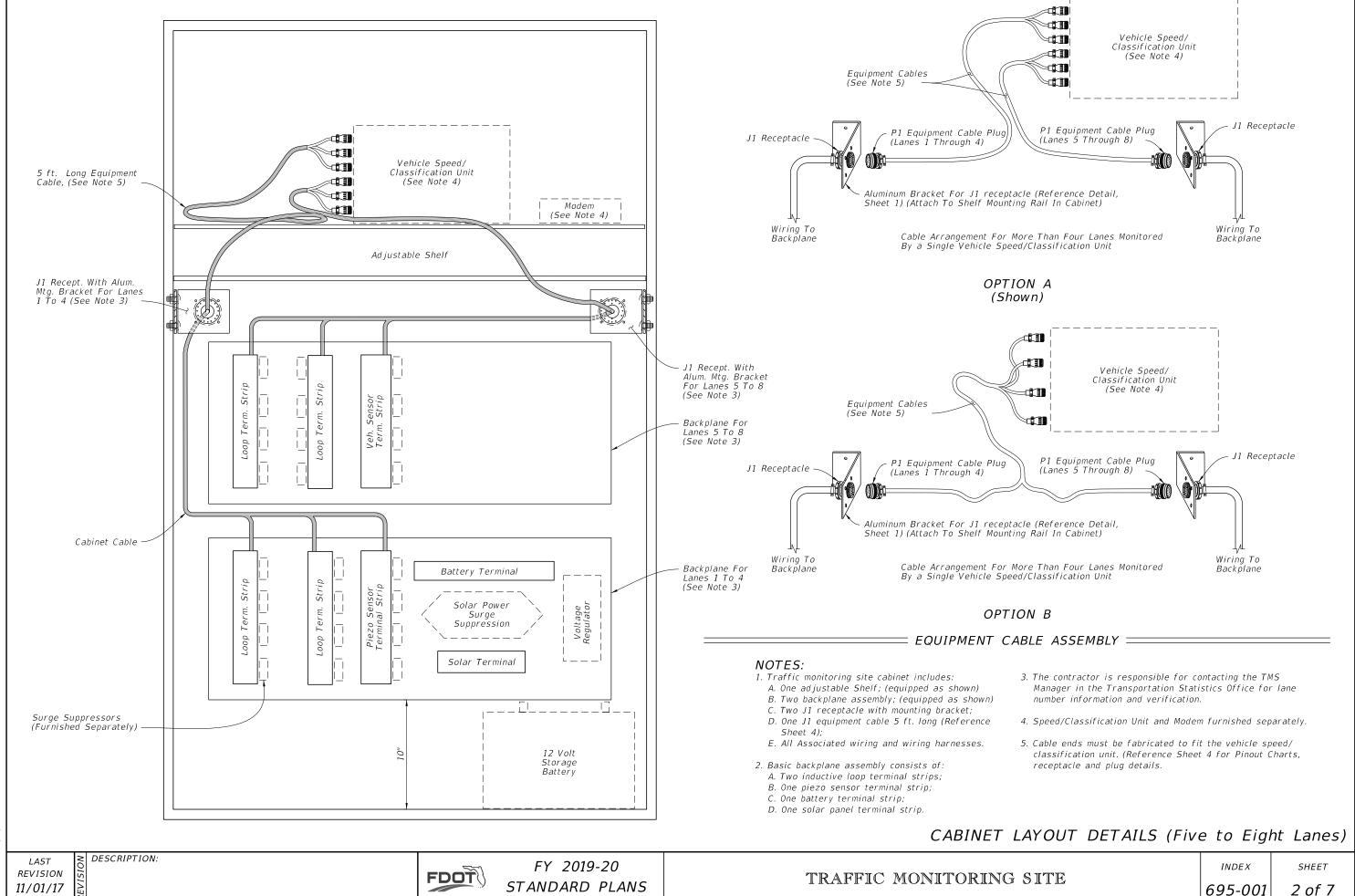
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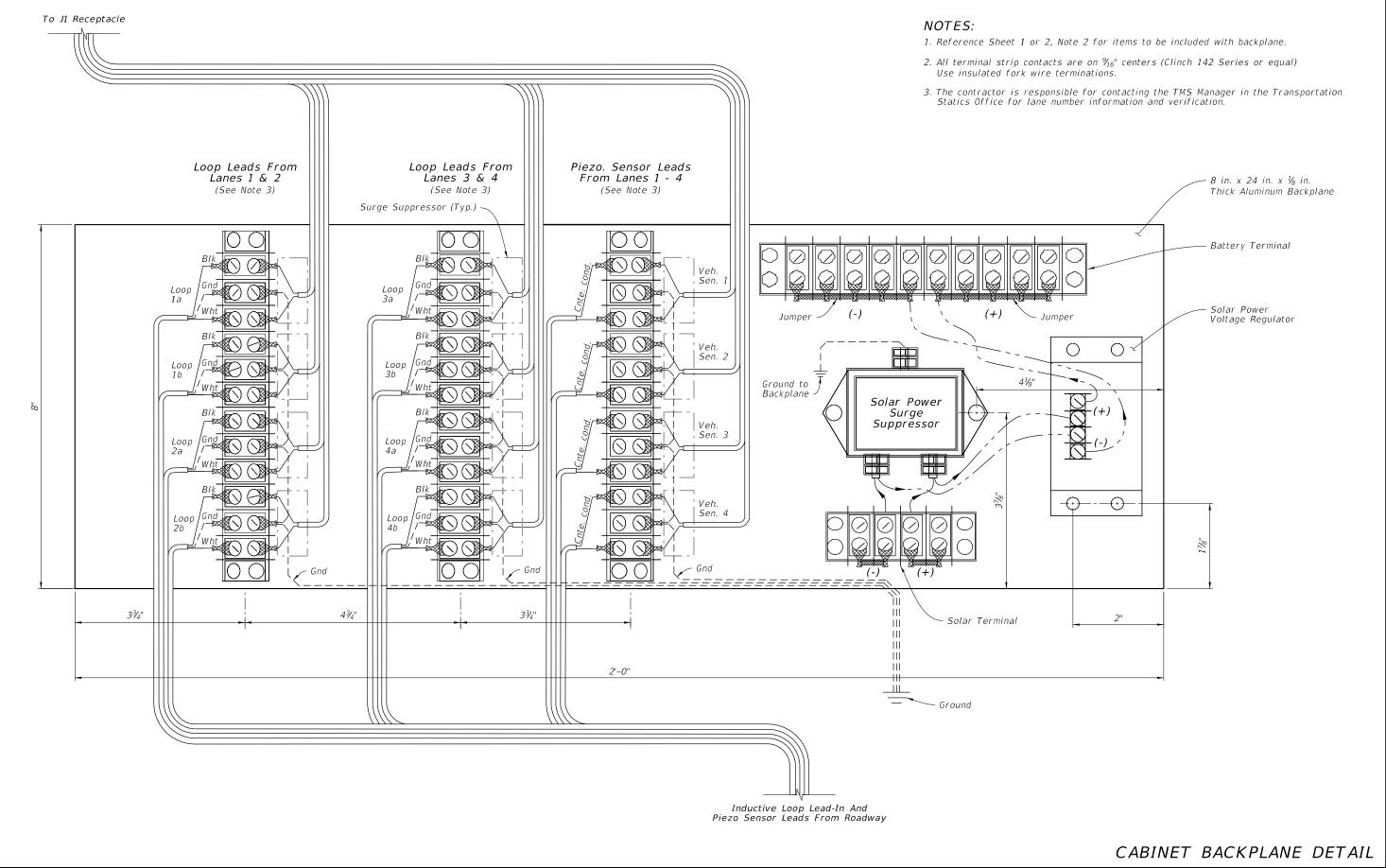
STANDARD PLANS

TRAFFIC MONITORING SITE

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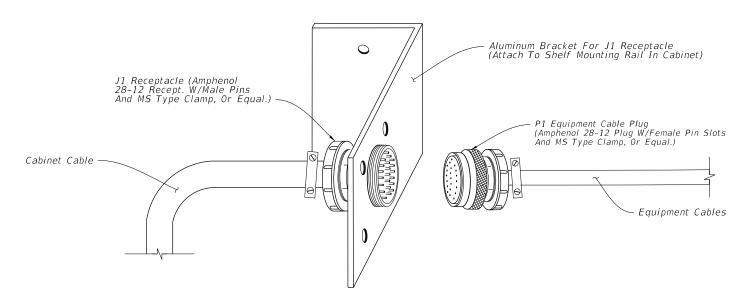
FDOT

FY 2019-20 STANDARD PLANS

TRAFFIC MONITORING SITE

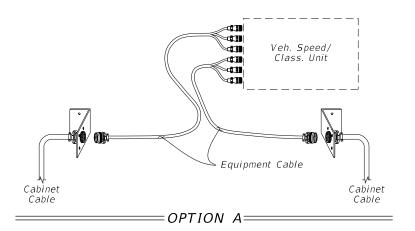
INDEX 695-001

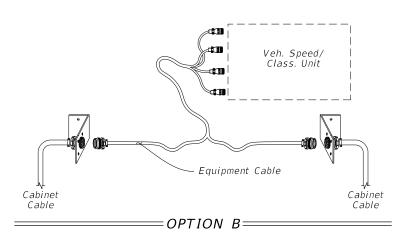
SHEET 3 of 7



	J1 RECEPTACLE PINOUT							
	26 Recessed Male Pins							
А	Loop 1a (5a) yellow							
В	Loop 1a (5a) purple							
С	Loop 1b (5b) gray							
D	Loop 1b (5b) pink							
Е	Loop 2a (6a) brown							
F	Loop 2a (6a) blue							
G	Loop 2b (6b) orange							
Н	Loop 2b (6b) tan							
J	Loop 3a (7a) white							
К	Loop 3a (7a) green							
L	Loop 3b (7b) red							
М	Loop 3b (7b) black							
N	Gnd							
Р	Loop 4a (8a) w/yellow							
R	Loop 4a (8a) w/purple							
S	Loop 4b (8b) w/gray							
Т	Loop 4b (8b) w/brown							
U	Piezo 1 (5) (+) w/blue							
V	Piezo 1 (5) sh w/orange							
W	Piezo 2 (6) (+) w/green							
Х	Piezo 2 (6) sh w/red							
Y	Piezo 3 (7) (+) w/black							
Z	Piezo 3 (7) sh w/red/blk							
а	Piezo 4 (8) (+) red/ green							
b	Piezo 4 (8) sh red/yellow							
d	Gnd red/black							

J1	J1 EQUIPMENT CABLE PLUG						
26 Female Pin Slots							
А	Loop 1a (5a)						
В	Loop 1a (5a)						
С	Loop 1b (5b)	] ].					
D	Loop 1b (5b)	To Unit					
Ε	Loop 2a (6a)	Connect To					
F	Loop 2a (6a)	Conn					
G	Loop 2b (6b)	E1e					
Н	Loop 2b (6b)						
N	Gnd						
J	Loop 3a (7a)						
К	Loop 3b (7b)						
L	Loop 3b (7b)	t					
М	Loop 3b (7b)	To Uni					
Р	Loop 4a (8a)	Sonnect To					
R	Loop 4a (8a)	Conr					
S	Loop 4b (8b)	Ele					
Т	Loop 4b (8b)						
d	Gnd						
U	Piezo 1 (5) (+)						
V	Piezo 1 sh						
W	Piezo 2 (6) (+)	Init					
Х	Piezo 2 sh	ct To					
Y	Piezo 3 (7) (+)	Connect To					
Z	Piezo 3 sh	Ct					
а	Piezo 4 (8) (+)						
b	Piezo 4 sh						





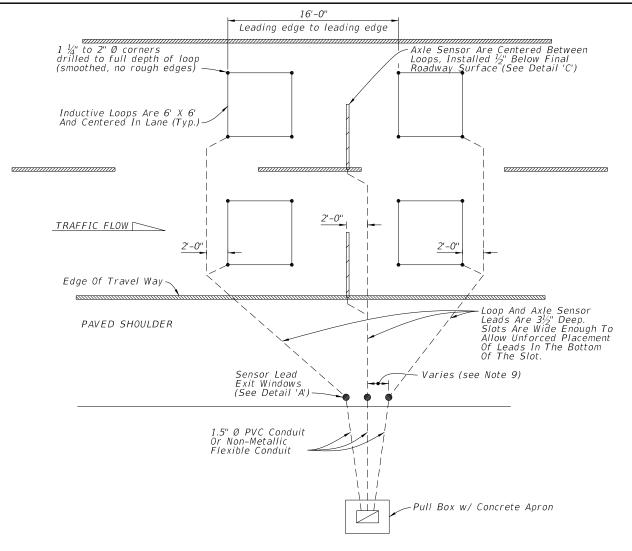
- 1. The contractor is responsible for contacting the TMS Manager in the Transportation Statistics Office for lane number information and verification.
- 2. The equipment cable can accommodate up to four lanes of inductive loop and piezo sensor inputs. (Reference Sheet 1 for cabinet layout)
- 3. For more than four lanes and up to eight lanes of inputs, the following options are available:
- A. Second Vehicle Speed/Class. Unit and separate equipment cable connecting to a second J1 receptacle; or
- B. Single Vehicle Speed/Class. Unit capable of up to eight lanes of inputs and a single equipment cable with split ends to fit two J1 receptacles. (Reference Sheet 2 detail)
- 4. Numbers in parenthesis in the pinout chart identify lane numbers when a second backplane for lanes 5 through 8 is required.
- 5. Cable Ends must be fabricated to fit the vehicle Speed/Classification Unit.

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DESCRIPTION:



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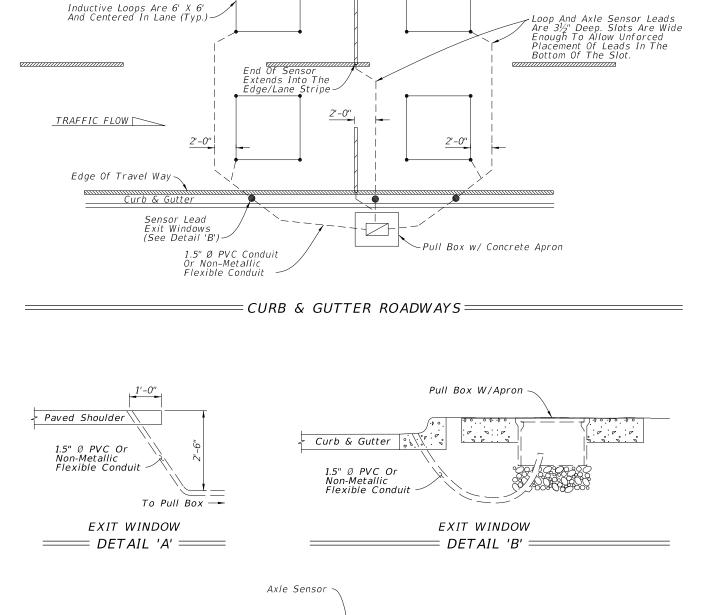


### =ROADWAYS WITH PAVED SHOULDERS =

### NOTES:

- 1. Install axle sensors and loops associated with axle sensors after placement of the friction course.
- 2. Cut a  $3\frac{1}{2}$ " deep slot for the Inductive loops. Loop slots will be cut wide enough to allow unforced placement of the wire into the bottom of the slot. Four turns of #14 AWG, place the IMSA 51-7 copper wire in the slot. Place short pieces of backer rod (2" to 3" in length) every 18" to 24" to hold the loop wire in the bottom of the slot.
- 3. Twist loop leads at the rate of 8 to 16 twists per foot. Loops that are within 150' of the cabinet, extend the twisted pair loop wire directly to the cabinet. For distances over 150', #14 IMSA 50-2 shielded lead-in cable must be spliced to the loop wire twisted pair at the first pull box to which the loop wire is pulled.
- 4. Marking will consist of two rounds of contrasting colored tape, one color for the lane number and the second color for the lead loop location in the lane. The first band closest to the cabinet will represent the lane number, one round of tape will be for lane 1 and two rounds will be lane 2, etc. The lead loop in lane one would have one round of tape and a second round of a contrasting colored tape for the lead loop in the lane. The trailing loop would not have a second contrasting colored band of tape.
- 5. See Index 635-001 for pull box and apron details.
- 6. All splices will be performed using splice kits designed for direct burial. Splice kits will include screw on wire connectors and a housing with sufficient sealant to fully encapsulate the spliced connections. Taped splices are not permitted.
- 7. Use a chalk line or string and paint to layout the position of the sensor and lead-in cable slots. Ensure saw cuts do not deviate more than 0.5 inches from the chalk line. Use a single blade or ganged blade saw wide enough to cut the axle sensor slot at full width in a single pass. Cutting two slots and chipping out roadway material between
- 8. All sensor slots and any cuts in the roadway will be thoroughly blown out to ensure there is no dust or debris prior to installation of sensors or leads.
- 9. Install Exit Windows at least 2' apart.

DESCRIPTION:

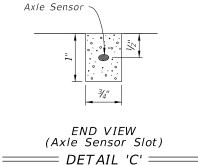


16'-0"

Leading edge to leading edge

-Axle Sensor Are Centered Between Loops, Installed ½" Below Final Roadway Surface (See Detail 'C')

1½" to 2" Ø corners drilled to full depth of loop (smoothed, no rough edges)



= TYPICAL FOR UP TO 4 LANES OF SENSOR LEADS PULLED TO ONE SIDE OF THE ROADWAY ==

LANE LAYOUT FOR TMS INDUCTIVE LOOP AND AXLE SENSOR

**REVISION** 11/01/17

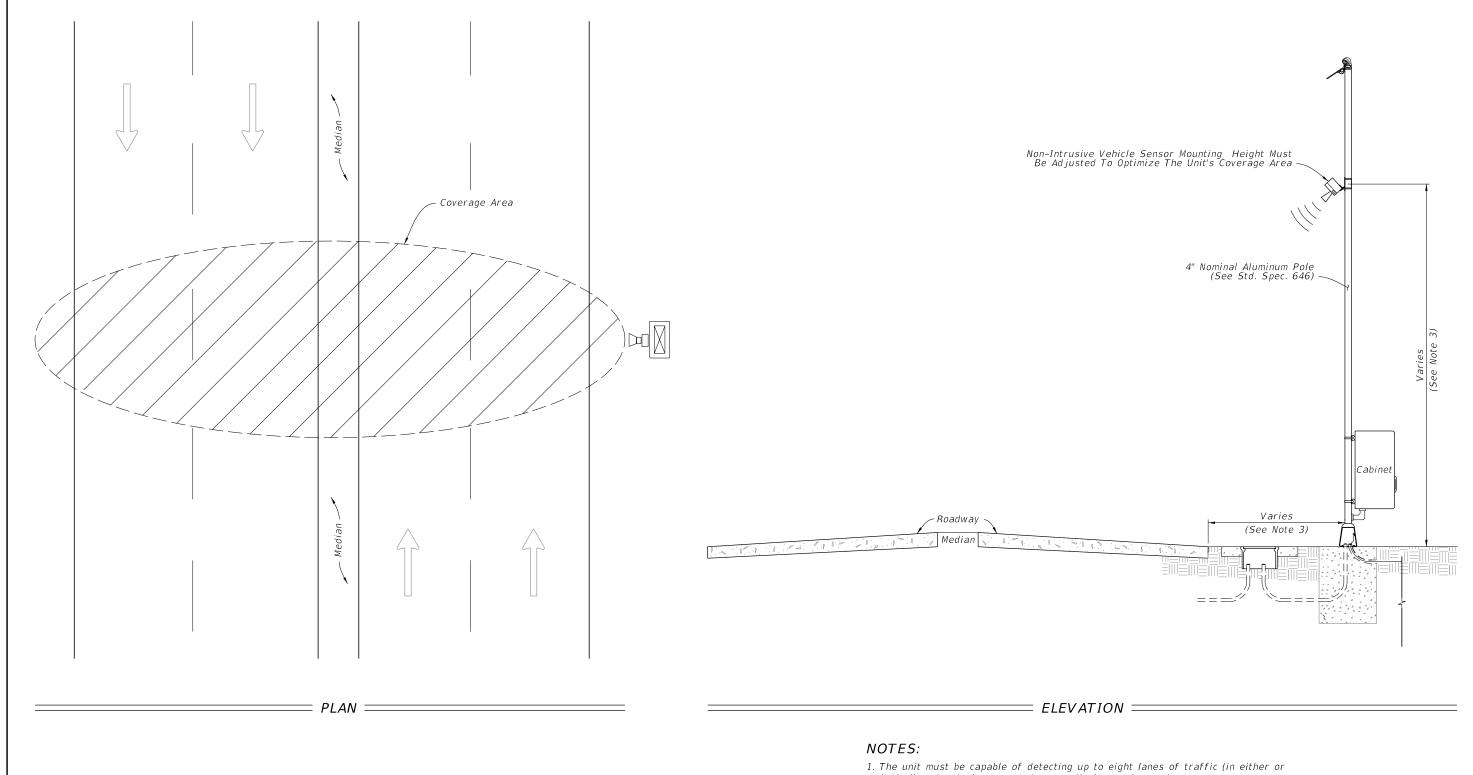
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- both directions) when mounted perpendicular to the roadway.
- 2. Coverage area of the unit is affected by the roadway geometry: distance from the travel lanes, median type and width, barrier walls, etc.
- 3. Mounting height of the unit and offset from the roadway must be determined on a site-by-site basis, in accordance with the manufacturer's recommended guidelines. Offset of pole must be greater than or equal to minimum clear zone requirements.

NON-INTRUSIVE VEHICLE SENSOR

REVISION 11/01/17

DESCRIPTION:

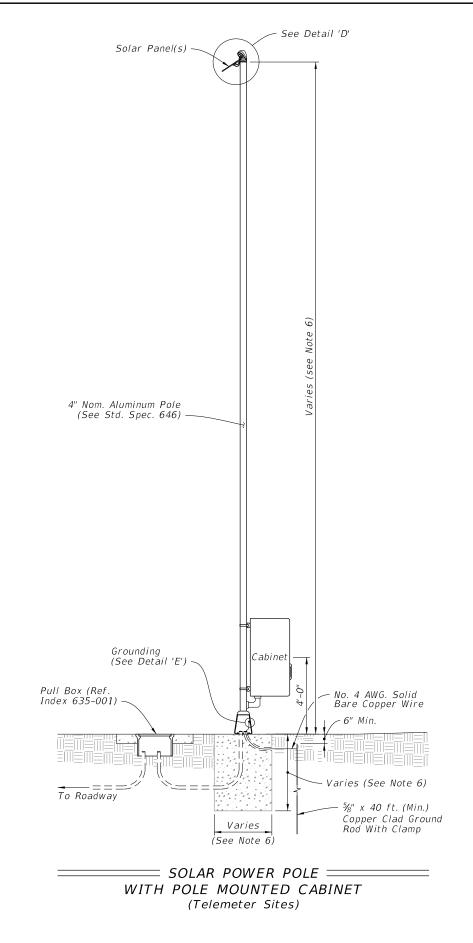
FDOT

FY 2019-20 STANDARD PLANS

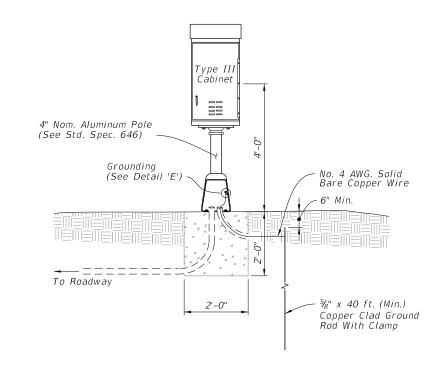
INDEX 695-001

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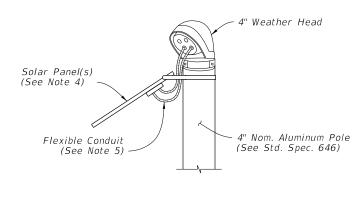
SHEET



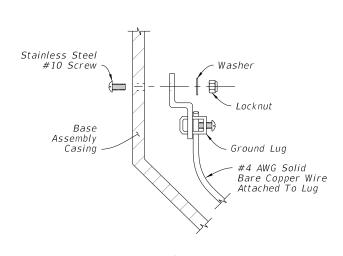
- 1. Cabinet installed per Index 676-010 except cabinet center will be 4 feet above grade.
- 2. Place pole in accordance with the Standard Specification 125.4 and 125.8.2.
- 3. Use #10 AWG stranded copper wire for Solar Panel Array installations, Red insulation is THHN or THWN for positive 12 volts wiring, Black insulation is THHN or THWN for negative, 12 volts wiring, Green insulation is THHN or THWN for ground bonding of the solar panel frame to the pole and earth.
- 4. Solar panel should be installed facing due south with angle of tilt equal to the sum of the following equation. The Latitude of the panel's location, multiplied by 0.76, plus 3.1 degrees. Equation expressed as (LAT)X(0.76)+(3.1°)
- 5. Encase all wiring from the weather head to the solar panel in outdoor flexible conduit.
- 6. Concrete Base Requirements:
- a. 4' poles: 2'-0" X 2'-0" wide, a depth of 2'-0"
- b. 12', 15' or 20' poles: 3'-0" X 3'-0" wide, a depth of 3'-0"
- c. 30' or 35' poles: 3'-0" X 3'-0" wide, a depth of 4'-0"







DETAIL 'D' =



DETAIL 'E' =

LAST **REVISION** 11/01/17

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SHEET

011 3	in Sheets 7, 0, and 9.								
	Cina		Centroid						
	Size a x h Local 'Yn'		Global 'Xn'	Global 'Yn'	'A'n	'X' _n x 'A' _n	'Y' _n x 'A' _n		
	(in. x in.)	(in.)	(in.)		(in.²)	(in.³)	(in.³)		
1	21 x 15	7.5	-10.5 - 1.5 - 1.5 = -13.5	7.5	315	-4,252.5	2,362.5		
2	21 x 15	7.5	10.5+1.5+1.5 = 13.5	7.5	315	+4,252.5	2,362.5		
3	24 x 24	12	-12-1.5 = -13.5	15+1+12 = 28	576	-7,776	16,128		
4	24 x 24	12	12+1.5 = 13.5	15+1+12 = 28	436	5,886	12,208		
5	24 x 12	6	-12-1.5 = -13.5	15+1+24+1+6 = 47	288	-3,888	13,536		
6	24 x 12	6	12+1.5 = 13.5	15+1+24+1+6 = 47	288	3,888	13,536		
				TOTALS	2,218	-1,890	60,133		

$$\Sigma (A'_n) = 2,218 \text{ in.}^2 = 15.4 \text{ ft.}^2$$

$$\Sigma ('X_{n}' \times 'A_{n}') = -1.890 \text{ in.}^3 = -1.09 \text{ ft.}^3$$

$$\Sigma ('Y_n' \times 'A_n') = 60,133 \text{ in.}^3 = 34.8 \text{ ft.}^3$$

$${}^{\prime}X_{C}^{\prime} = -\frac{\Sigma \left( {}^{\prime}X_{n}^{\prime}X_{n}^{\prime}A_{n}^{\prime} \right)}{\Sigma {}^{\prime}A_{n}^{\prime}} = -0.1 \text{ ft.} \qquad {}^{\prime}Y_{C}^{\prime} = \frac{\Sigma \left( {}^{\prime}Y_{n}^{\prime}X_{n}^{\prime}A_{n}^{\prime} \right)}{\Sigma {}^{\prime}A_{n}^{\prime}} = 2.26 \text{ ft.}$$

$${}^{\prime}Y_C' = \frac{\sum \left( {}^{\prime}Y_N' x {}^{\prime}A_N' \right)}{\sum {}^{\prime}A_N'} = 2.26 \ ft.$$

STEP 2: Determine the height 'H' from groundline to the centroid of the individual sign or sign cluster.

Assume: 'B' = 1 ft., 'C' = 7 ft.

Calculated:  $X'_{c} = -0.1 \text{ ft., } 'Y'_{c} = 'D' 2.26 \text{ ft.}$ 

$$'H' = 'B' + 'C' + 'D' = 10.26 \ ft. ==> \boxed{USE \ 11 \ ft.} \qquad \Sigma ('A'_n') = 15.4 \ ft.^2 ==> \boxed{USE \ 16 \ ft.^2}$$

STEP 3: Refer to the Aluminum Column (Post) Selection Tables and find the intersection point. See Sheet 3.

	ALUMINUM COLUMN (DOCT) CELECTION TABLE													
	ALU	ALUMINUM COLUMN (POST) SELECTION TABLE												
							H' (F							
		8 ft	9 ft	10 ft			13 ft		15 ft	16 ft	17 ft	18 ft	19 ft	
	3 sf	2	2.5	2.5	2.5	3	3	3	3	3.5	3.5	3.5	3.5	3.5
	4 sf	2.5	2.5	3	3	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5
	5 sf	2.5	3	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4
	6 sf	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4
	7 sf	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4
	8 sf	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4
	9 sf	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4
_	10 sf	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4.5	4.5
(SF.	11 sf	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4.5	4.5	4.5
	12 sf	3.5	3.5	3.5	4	4	4	4	4	4	4	4.5	4.5	4.5
AREA	13 sf	3.5	3.5	4	4	4	4	4	4	4	4.5	4.5	4.5	5
IRE	14 sf	3.5	3.5	4	4	4	4	4	4	4.5	4.5	4.5	5	5
	15 sf	3.5	4	4	4	4	4	4	4.5	4.5	4.5	5	5	5
7 <i>3</i>	16 sf	3.5	4	4	4	4	4	4	4.5	4.5	5	5	5	6
PANEL	17 sf	4	4	4	4	4	4	4.5	4.5	4.5	5	5	6	6
9	18 sf	4	4	4	4	4	4.5	4.5	4.5	5	5	5	6	6
7	19 sf	4	4	4	4	4	4.5	4.5	4.5	5	5	6	6	6
TOTAL	20 sf	4	4	4	4	4.5	4.5	4.5	5	5	5	6	6	6
1 2	21 sf	4	4	4	4	4.5	4.5	5	5	5	6	6	6	6
' '	22 sf	4	4	4	4.5	4.5	4.5	5	5	6	6	6	6	6
	23 sf	4	4	4	4.5	4.5	5	5	5	6	6	6	6	6
	24 sf	4	4	4.5	4.5	4.5	5	5	6	6	6	6	6	6
	25 sf	4	4	4.5	4.5	5	5	5	6	6	6	6	6	8
	26 sf	4	4.5	4.5	4.5	5	5	5	6	6	6	6	8	8
	27 sf	4	4.5	4.5	4.5	5	5	6	6	6	6	6	8	8
	28 sf	4	4.5	4.5	5	5	5	6	6	6	6	6	8	8
	29 sf	4.5	4.5	4.5	5	5	6	6	6	6	6	8	8	8
	30 sf	4.5	4.5	5	5	5	6	6	6	6	6	8	8	8

For  $'H' = 11 \text{ ft.}, Area = 16 \text{ ft.}^2$ 

- Refer to the Aluminum Column (Post) Selection Table, from Sheet 3 and shown here for reference.
- To determine the required post size, find the intersection of the row labeled "16 SF" and the column labeled "11 FT". For the example the intersection value is "4" (4" OD).
- In the Column (Post) and Foundation Table, the value "4" shows the design requires a 4.0" diameter and 1/4" thick Aluminum Column (Post) and a 2.0' diameter and 3.5' deep Concrete Foundation and 3.0' Stub.

SHEET	CONTENTS
1	General Notes and Design Example
2	Design Example - Centroid
3	Column and Foundation Tables
4	Slip Base and Foundation Details
5	Driven Post and Soil Plate Detail
6	Wind Beam Connection
7, 8 & 9	Frequently Used Sign Clusters

### GENERAL NOTES:

1. Shop Drawings:

This Index is considered fully detailed. Submit Shop Drawings only for minor modifications not detailed in the Plans.

- 2. Aluminum Sign, Wind Beams and Column (Post) Materials:
  - A. Aluminum Plates: ASTM B209, Alloy 6061-T6
  - B. Aluminum Bars and Extruded Shapes: ASTM B221, Alloy 6061-T6
  - C. Aluminum Structural Shapes: ASTM B308 Alloy 6061-T6
  - D. Cast Aluminum: ASTM B26 Allov A356-T6
  - E. Aluminum Weld Material: ER 5556 or 5356
- 3. Sign Mounting Bolts, Nuts and Washers:
  - A. Aluminum Button Head and Flat Head Bolts: ASTM F468 Alloy 2024-T4
  - B. Aluminum Hex Nuts: ASTM F467 Alloy 6061-T6 or 6262-T9
  - C. Aluminum Washers: ASTM B221, Alloy 7075-T6
- 4. Stainless Steel Bolts, Nuts and Washers may be used in lieu of the Aluminum button head and flat head bolts as follows:
- A. Stainless Steel Bolts: ASTM F 593 Alloy Group 2, Condition A, CW1 or SH1
- B. Stainless Steel Nuts: ASTM F594
- 5. Sign Column (Post) Bolts, Nuts and Washers:
  - A. Galvanized U-Bolt (Column): ASTM A449 or ASTM A193 B7 according to ASTM F2329 with double nuts.
  - B . Aluminum Bolts (Sleeve): ASTM F468, Alloy 6061-T6 or 2024-T4 with Hex Nuts F467 6061-T6 or 6262-T9 and Washers B221, Alclad 2024-T4
- C. Galvanized High Strength Hex Head Bolts (BaseBolts): ASTM F3125, Grade A325, Type 1
- D. Galvanized Hex Nuts: ASTM A563 Grade DH
- E. Galvanized Washers: ASTM F436
- F. Galvanized Bolts (Sleeve): ASTM A307 with Galvanized Hex Nuts and Washers
- - A. Aluminum Fasteners: Anodic coating (0.0002 inches min.) and chromate sealed
  - B. High Strength Steel Bolts Nuts and Washers: ASTM F2329
- C. All other steel items (excluding stainless steel): Hot-dip Galvanize ASTM A123
- D. Repair damaged galvanizing in accordance with Specification 562
- 7. BREAKAWAY SUPPORTS REQUIREMENTS: Install non-frangible aluminum column (post) (larger than  $3\frac{1}{2}$ ") with breakaway supports as shown on Sheet 4. Signs shielded by barrier wall or guardrail do not require breakaway support.

STEP 4: For sign assemblies with signs oriented in two directions, only the sign with the largest area should be analyzed to determine the Column (Post) requirements.

GUIDE TO USE THIS INDEX

GENERAL NOTES AND DESIGN EXAMPLE

**REVISION** 11/01/18

DESCRIPTION:

**FDOT** 

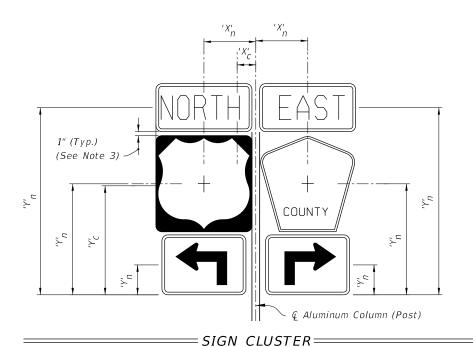
FY 2019-20 STANDARD PLANS

SINGLE COLUMN GROUND SIGNS

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SHEET

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$$'X'_{C} = \frac{\sum \left( \begin{array}{ccc} X'_{n} \times A'_{n} \right)}{\sum A'_{n}} \qquad C' = Y'_{C} = \frac{\sum \left( \begin{array}{ccc} Y'_{n} \times A'_{n} \right)}{\sum A'_{n}} \end{array}$$

 $'A'_n = Area of individual sign$ 

 $^{\prime}B^{\prime}$  = Height of the edge of pavement from the mounting elevation

 ${}^{\prime}{}C^{\prime}$  = Height of the the bottom of the sign or cluster from the edge of pavement elevation

 $^{\prime}D^{\prime}$  = Height of the centroid of the sign or cluster from the bottom of the sign or cluster

h = Individual sign height

'H' = Height of sign or cluster centroid from groundline

a = Individual sign width

DESCRIPTION:

 $'X'_{C} = Centroid\ horizontal\ location\ of\ sign\ or\ cluster\ from\ \ \ \ Aluminum\ Column\ (Post)$ 

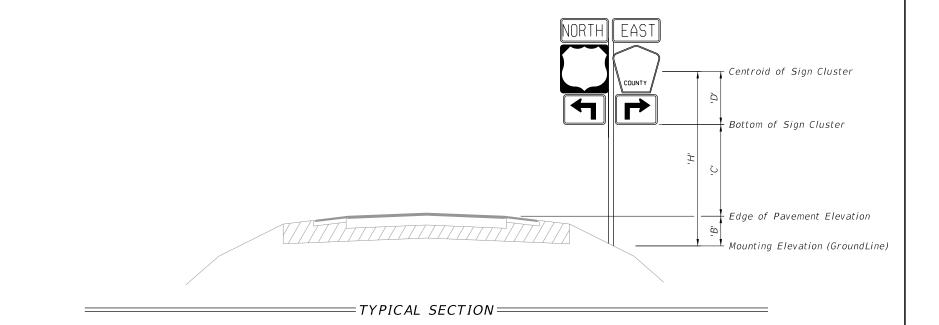
 $'Y'_{c}$  = Centroid height of sign or cluster from bottom of sign cluster

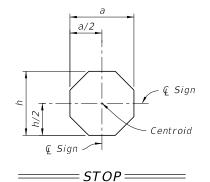
 $'X'_n = Individual \ sign \ centroid \ horizontal \ location \ from \ \ \ \ Aluminum \ \ Column \ (Post)$ 

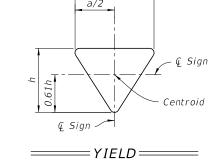
 $'Y'_n = Individual Sign centroid height from bottom of sign cluster$ 

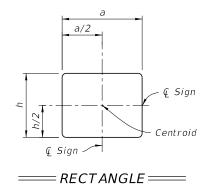
- 1. For 'B' & 'C' see Index 700-101 and Roadway Plans.
- 2. Do not exceed an area of 30 SF or a width of 60 inches for a sign or a sign cluster, including rotated sign panels.
- 3. Vertical sign spacing (1" shown on Sign Cluster detail) also applies to rotated signs.

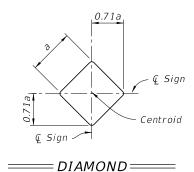
=CALCULATION OF SIGN CLUSTER CENTROID==

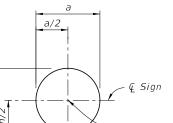


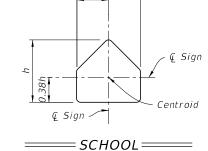


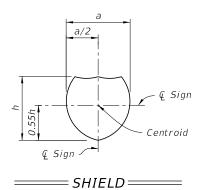


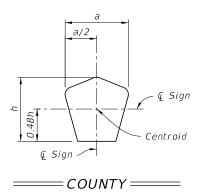












DESIGN EXAMPLE - CENTROID

REVISION 11/01/18

FDOT

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===== RAILROAD

SINGLE COLUMN GROUND SIGNS

INDEX 700-010

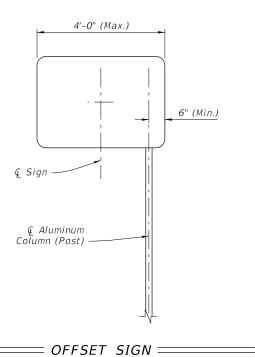
SHEET 2 of 9

		ALUMINUM COLUMN (POST) SELECTION TABLE (O.D. in.)												
		'H' (FT)												
		8 ft	9 ft	10 ft	11 ft	12 ft	13 ft	14 ft	15 ft	16 ft	17 ft	18 ft	19 ft	20 ft
	3 sf	2	2.5	2.5	2.5	3	3	3	3	3.5	3.5	3.5	3.5	3.5
	4 sf	2.5	2.5	3	3	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5
	5 sf	2.5	3	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4
	6 sf	3	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4
	7 sf	3	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4
	8 sf	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4
	9 sf	3.5	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4
	10 sf	3.5	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4.5	4.5
	11 sf	3.5	3.5	3.5	3.5	4	4	4	4	4	4	4.5	4.5	4.5
3F)	12 sf	3.5	3.5	3.5	4	4	4	4	4	4	4	4.5	4.5	4.5
AREA (SF)	13 sf	3.5	3.5	4	4	4	4	4	4	4	4.5	4.5	4.5	5
NEA	14 sf	3.5	3.5	4	4	4	4	4	4	4.5	4.5	4.5	5	5
1	15 sf	3.5	4	4	4	4	4	4	4.5	4.5	4.5	5	5	5
IEL	16 sf	3.5	4	4	4	4	4	4	4.5	4.5	5	5	5	6
PANEL	17 sf	4	4	4	4	4	4	4.5	4.5	4.5	5	5	6	6
	18 sf	4	4	4	4	4	4.5	4.5	4.5	5	5	5	6	6
TOTAL	19 sf	4	4	4	4	4	4.5	4.5	4.5	5	5	6	6	6
7	20 sf	4	4	4	4	4.5	4.5	4.5	5	5	5	6	6	6
	21 sf	4	4	4	4	4.5	4.5	5	5	5	6	6	6	6
	22 sf	4	4	4	4.5	4.5	4.5	5	5	6	6	6	6	6
	23 sf	4	4	4	4.5	4.5	5	5	5	6	6	6	6	6
	24 sf	4	4	4.5	4.5	4.5	5	5	6	6	6	6	6	6
	25 sf	4	4	4.5	4.5	5	5	5	6	6	6	6	6	8
	26 sf	4	4.5	4.5	4.5	5	5	5	6	6	6	6	8	8
	27 sf	4	4.5	4.5	4.5	5	5	6	6	6	6	6	8	8
	28 sf	4	4.5	4.5	5	5	5	6	6	6	6	6	8	8
	29 sf	4.5	4.5	4.5	5	5	6	6	6	6	6	8	8	8
	30 sf	4.5	4.5	5	5	5	6	6	6	6	6	8	8	8

	FOUNDATION TABLE									
Column (	Column (Post) Foundation Alternatives									
Size		Driven	Post *	Con	crete (Class	: I)				
Outside	Wall	Embedment	Depth (ft)	Diameter	Embedment	Stub				
Diameter (in)	Thk. (in)	without Soil Plate	with Soil Plate	(ft)	Depth (ft)	Length (ft)				
2.0	1/8	4.5	2.5							
2.5	1/8	5.0	3.0							
3.0	1/8	5.0	3.5							
3.5	³ / ₁₆	6.0	4.5							
4.0	1/4			2.0	3.5	3.0				
4.5	1/4			2.0	4.0	3.0				
5.0	1/4			2.0	4.5	3.0				
6.0	1/4			2.0	5.0	3.0				
8.0	1/4			2.0	5.5	3.0				

### * INSTALLING FRANGIBLE COLUMN SUPPORTS:

Columns (posts)  $3\frac{1}{2}$ " O.D. and less are considered frangible and may be installed either by driving the post or setting the posts in preformed holes. Backfill preformed holes with suitable material tamped in layers not thicker than 6" (to provide adequate compaction) or filled with flowable fill or bagged concrete.



## NOTE:

- 1. For offset sign placement see Index 700-101.
- 2. For signs with widths greater than 4' see Index 700-011.
- 3. Offset signs with driven posts require a soil plate.

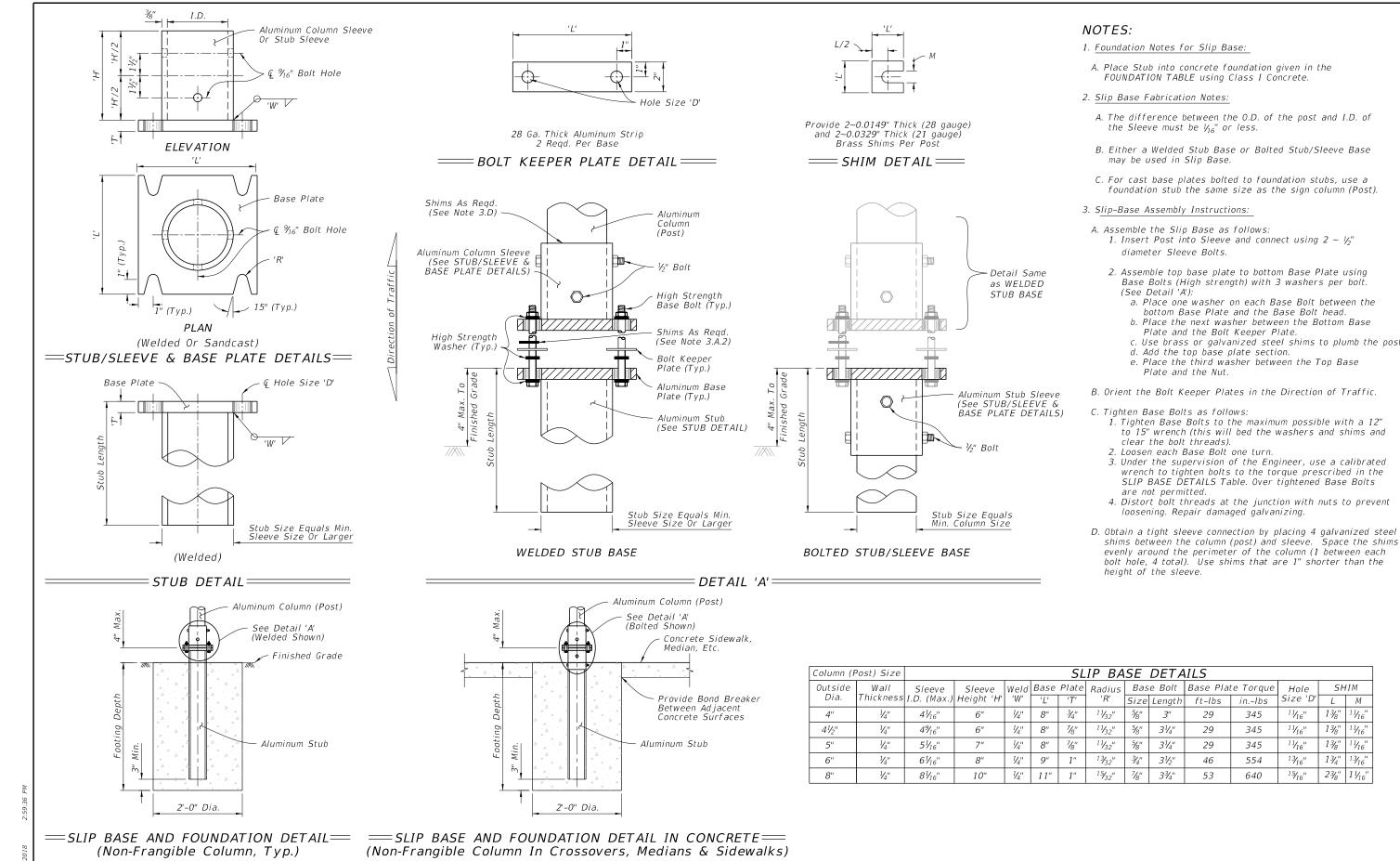
### COLUMN AND FOUNDATION TABLES

LAST REVISION 11/01/17

≥ DESCRIPTION:

FD

FY 2019-20 STANDARD PLANS



SLIP BASE AND FOUNDATION DETAILS

ft-lbs

29

29

29

46

53

31/4"

31/4"

31/2"

33/4"

in.-Ibs

345

345

345

554

bottom Base Plate and the Base Bolt head.

Plate and the Bolt Keeper Plate.

Plate and the Nut.

**REVISION** 11/01/18

DESCRIPTION:

**FDOT** 

FY 2019-20 STANDARD PLANS

SINGLE COLUMN GROUND SIGNS

INDEX 700-010

SHEET

4 of 9

SHIM

L | M

13/8" | 11/16"

13/4" 13/16"

11/16"

1 ½16"

13/8"

13/8"

15/16" 23/8" 11/16"

Hole

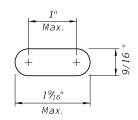
Size 'D'

¹1/₁₆"

1½16"

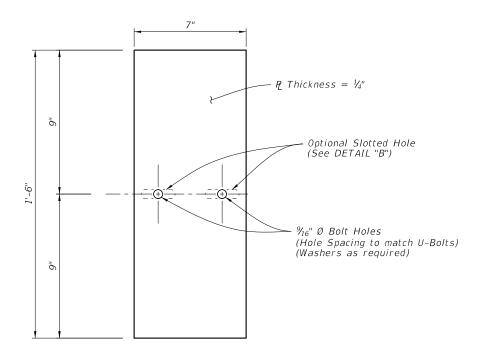
¹½16"

¹³/₁₆"



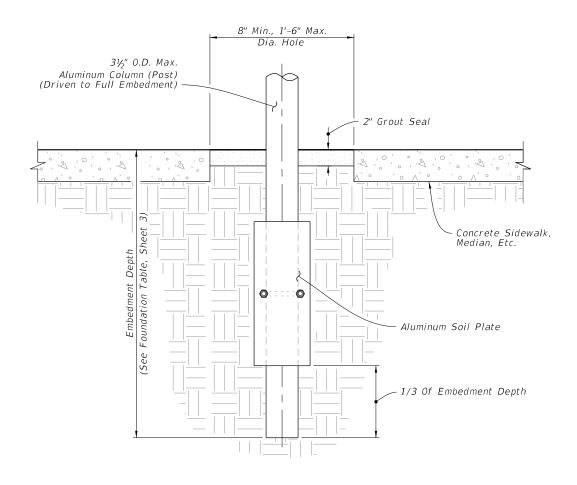
Optional Slotted Holes

= DETAIL "B" ==



= ALUMINUM SOIL PLATE DETAIL=====

3½" O.D. Max. Aluminum Column (Post) Hole Aluminum Soil Plate 2" Thick Grout Seal PLANConcrete Sidewalk, Median, Etc.



ELEVATION

= DRIVEN POST DETAIL ===

(Frangible Post In Crossovers, Medians & Sidewalks)

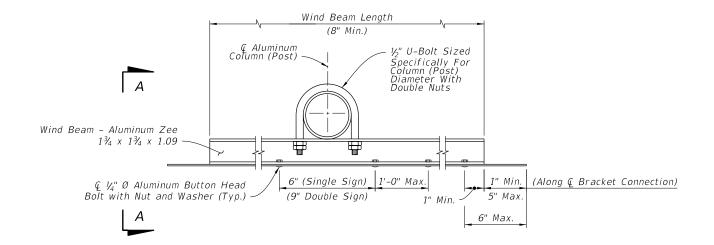
DRIVEN POST AND SOIL PLATE DETAIL

11/01/18

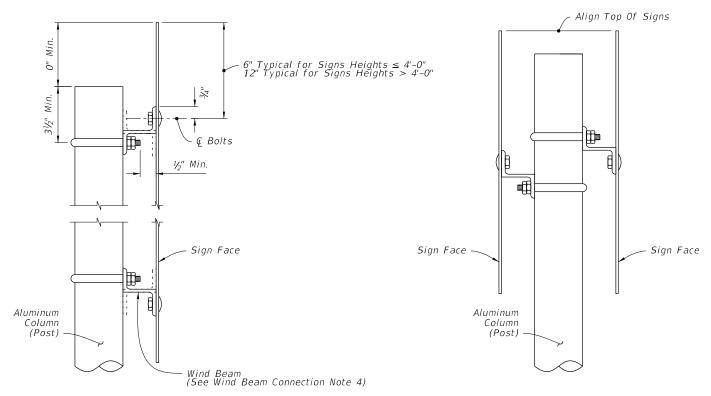
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS



### = WIND BEAM CONNECTIONS DETAILS =



NOTE: Use the area and the centroid location of the largest sign to determine aluminum column (post) size.

BACK-TO-BACK SIGN DETAIL

### NOTES:

- 1. 5/16" Ø Stainless Steel Hex Head Bolts with Flat Washer under Head and Washer under Nut may be used in lieu of 1/4" Ø Aluminum Button or Flat Head Bolts.
- 2. Use Nylon washers (provided by the sheeting supplier) under the button bolt heads to protect sign sheeting.
- 3. Slots up to 2" long are allowed in wind beams to accommodate U-Bolts for varying Column (Post) diameters.
- 4. Wind beams may be oriented in either direction.
- 5. For rectangular signs greater than 66" install a third wind beam evenly spaced between the top and bottom wind beams. For rectangular signs up to 12" in height, use only one wind beam at & Sign.

= VIEW A-A ==

WIND BEAM CONNECTION

LAST **REVISION** 11/01/18

DESCRIPTION:

FDOT

SINGLE SIGN DETAIL

FY 2019-20 STANDARD PLANS

SINGLE COLUMN GROUND SIGNS

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SHEET

	Size	Area	Total Area	Centroid
ONE WAY	36×12	3.00 SF		
			6.31 SF	
STOP	24x24	3.31 SF		
	Size	Area	Total Area	Centroid
ONE WAY	36×12	3.00 SF	-	
STOP	30x30	5.18 SF	8.18 SF	1.92 Ft.
	Size	Area	Total Area	Centroid
ONE WAY				
UNE WAY	36×12	3.00 SF	10.46.65	
STOP	36x36	7.46 SF	10.46 SF	2.10 Ft. ———————
	Size	Area	Total Area	Centroid
ONE WAY	36×12	3.00 SF	-	
			16.25 SF	
STOP	48x48	13.25 SF		
	Size	Area	Total Area	Centroid
STOP	24x24	3.31 SF	6.31 SF	
DIVIDED	24x18	3.00 SF	-	
	Size	Area	Total Area	Centroid
STOP	30x30	5.18 SF	10.18 SF	
DIVIDED	30x24	5.00 SF		
	Size	Area	Total Area	Centroid
STOP	36x36	7.46 SF		 2.55 Ft. 
DIVIDED	30x24	5.00 SF		

	Size	Area	Total Area	Centroid
ONE WAY.	36×12	3.00 SF	-	
STOP	30×30	5.18 SF	13.18 SF	
DIVIDED	30x24	5.00 SF		
	Size	Area	Total Area	Centroid
ONE WAY	36x12	3.00 SF	_	
STOP	36x36	7.46 SF	15.46 SF	3.15 Ft.
DIVIDED	30x24	5.00 SF		
	Size	Area	Total Area	Centroid
JCT	21x15	2.19 SF		
27	24x24	4.00 SF	6.19 SF 	1.60 Ft.
	Size	Area	Total Area	Centroid
JCT	21x15	2.19 SF	7.19 SF	 1.52 Ft.
301	30x24	5.00 SF		
	Size	Area	Total Area	Centroid
BUSINESS OR EAST	24×12	2.00 SF		
27 27	24x24	4.00 SF	6.00 SF	1.53 Ft.
	Size	Area	Total Area	Centroid
BUSINESS OR EAST	24x12	2.00 SF		
301 301	30x24	5.00 SF	7.00 SF	
	Size	Area	Total Area	Centroid
BUSINESS OR EAST	30x15	3.13 SF		
301 301	30x24	5.00 SF	8.13 SF	1.66 Ft.

	Size	Area	Total Area	Centroid
27	24x24	4.00 SF	6.19 SF	1.73 Ft.
<b> </b>	21x15	2.19 SF		
	Size	Area	Total Area	Centroid
27	30×24	5.00 SF	7.19 SF	1.81 Ft.
<b>+</b>	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
BUSINESS OR EAST	24×12	2.00 SF		
27 27	24×24	4.00 SF	8.19 SF	2.26 Ft.
<b>→</b>	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
BUSINESS OR EAST	24×12	2.00 SF		
301 301	30x24	5.00 SF	9.19 SF	2.27 Ft.
$\rightarrow$	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
BUSINESS EAST	30×15	3.13 SF		
301 301	30x24	5.00 SF	10.32 SF	2.49 Ft.
	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
EAST	24x12	2.00 SF		
BUSINESS	24×12	2.00 SF	<u> </u> 	
27	24x24	4.00 SF	10.19 SF	2.80 Ft. 
<b>-&gt;</b>	21×15	2.19 SF		
	I			

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	Size	Area	Total Area	Centroid
	3120	Aica	, otal Alea	CCHEFOIU
EAST	24x12	2.00 SF		
FUONESS			_	
BUSINESS	24x12	2.00 SF	<u> </u>	
600			11.19 SF	2.76 Ft.
[301]	30x24	5.00 SF		
			-	
	21 v 1 F	2.19 SF		
	21x15	2.19 3F		
	Size	Area	Total Area	Centroid
EAST	30×15	3.13 SF	-	
LASI	30,113	3.13 31	-	
BUSINESS	30×15	3.13 SF	1	
			13.45 SF	2 16 54
301	30x24	5.00 SF	13.45 5F	3.16 Ft. 
30 1	30,24	3.00 SF		
<b>  →</b>	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
JCT	21x15	2.19 SF		
			3.90 SF	1.57 Ft.
LEON	18x18	171.65		
\ 56 county	10110	1.71 SF		
	Size	Area	Total Area	Centroid
	3128	Aiea	TOTAL ALEA	CEILLIOIU
JCT	21x15	2.19 SF		
			5.22 SF	1.72 Ft.
LEON	24x24	20255		
S6 COUNTY	27724	3.03 SF		
	Size	Area	Total Area	Centroid
	3,20	7.17.04	, otal Alea	55 514
JCT	21x15	2.19 SF		
			6.95 SF	1.87 Ft.
LEON 56	30x30	4.76 SF		
COUNTY	30,30	4./0 31		
COONT				

	C:		T	C t i d
	Size	Area	Total Area	Centroid
LEON 56 COUNTY	18×18	1.71 SF	3.90 SF	- — — — — — — — — — — — — — — — — — — —
	21×15	2.19 SF		- — — — — —
	Size	Area	Total Area	Centroid
LEON 56 COUNTY	24x24	3.03 SF	5.22 SF	1.62 Ft.
	21x15	2.19 SF		
	Size	Area	Total Area	Centroid
	3.20	, ca	. 563. 711.64	CCMC, OIG
LEON 56 COUNTY	30x30	4.76 SF	6.95 SF	1.97 Ft.
	21x15	2.19 SF		
	Size	Area	Total Area	Centroid
ТО	24x12	2.00 SF	-	
EAST	24x12	2.00 SF		
NTERSTATE 75	24x24	3.20 SF	9.39 SF	2.87 Ft.
	21x15	2.19 SF	-	
	Size	Area	Total Area	Centroid
ТО	24x12	2.00 SF		
EAST	24x12	2.00 SF	-	
NTERSTATE 295	30x24	3.99 SF	10.18 SF	2.84 Ft.
<b>—</b>	21x15	2.19 SF		

	Size	Area	Total Area	Centroid
ТО	30×15	3.13 SF		
EAST	30×15	3.13 SF		
NTERSTATE 295	30x24	3.99 SF	12.44 SF	3.26 Ft.
	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
JCT	21×15	2.19 SF	- 	
INTERCTATE			5.39 SF	1.75 Ft.
75	24x24	3.20 SF		
	Size	Area	Total Area	Centroid
JCT	21×15	2.19 SF		
NTERSTATE 295	30x24	3.99 SF	6.18 SF	1.67 Ft.
	Size	Area	Total Area	Centroid
EAST TO	24×12	2.00 SF		
NTERSTATE OR NTERSTATE 75	24x24	3.20 SF	5.20 SF	1.67 Ft.
	Size	Area	Total Area	Centroid
EAST TO	24×12	2.00 SF		
NIERSTATE 295	30x24	3.99 SF	5.99 SF	1.60 Ft.
	Size	Area	Total Area	Centroid
EAST TO	30×15	3.13 SF		
NIERSTATE OR NIERSTATE 295	30x24	3.99 SF	7.12 SF	1.81 Ft. 
	Size	Area	Total Area	Centroid
EAST TO	30×15	3.13 SF	1	
NTERSTATE OR NTERSTATE 75	36x36	7.20 SF	10.33 SF	2.27 Ft.

≥ DESCRIPTION: LAST REVISION 11/01/17

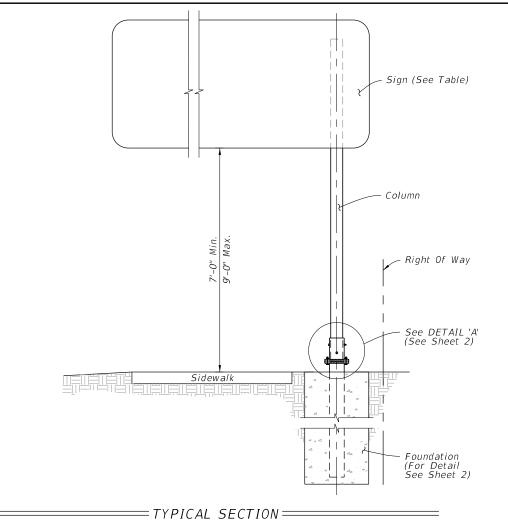
	Size	Area	Total Area	Centroid
EAST T	O 30x15	3.13 SF		
OR INTERSTATE INTER	STATE		12.12 SF	2.18 Ft.
295 29	95) 45x36	8.99 SF		
	Size	Area	Total Area	Centroid
EAST T	O 24x12	2.00 SF		
75 Times	24x24	3.20 SF	7.39 SF	2.30 Ft.
	21x15	2.19 SF		
	Size	Area	Total Area	Centroid
EAST T	O 24x12	2.00 SF		
	30x24	3.99 SF	8.18 SF	2.31 Ft.
	21x15	2.19 SF		
	Size	Area	Total Area	Centroid
EAST	O 30x15	3.13 SF		
NITERSTATE OR NITER 29	30x24	3.99 SF	9.31 SF	
	21×15	2.19 SF		
	Size	Area	Total Area	Centroid
THE OR THE	30×30	4.69 SF	6.69 SF	
AHEAD 200	24x12	2.00 SF		
	Size	Area	Total Area	Centroid
THE OR THE	30×30	4.69 SF	8.44 SF	
AHEAD 200	30x18	3.75 SF	_	
	Size	Area	Total Area	Centroid
THE OR THE	36x36	6.75 SF	10.50 SF	 2.06 Ft. 
AHEAD 200	30×18	3.75 SF		
DESCRIPTION:				

30X30 4.69 SF	 Ft.
24X12 2.00 SF	
Size Area Total Area Centr	oid
30X30 4.69 SF	— — — – Ft.
30X18 3.75 SF	
Size Area Total Area Centr	oid
36X36 6.75 SF	 Ft.
30X18 3.75 SF	
Size Area Total Area Centr	oid
30X30 6.25 SF 8.25 SF 2.28	 Ft.
AHEAD 24X12 2.00 SF	
Size Area Total Area Centr	oid
36X36 9.00 SF 12.75 SF 2.84	 Ft.
AHEAD 30X18 3.75 SF	
Size Area Total Area Centr	oid
30X30 6.25 SF	 Ft.
35 MPH 24X24 4.00 SF	
Size Area Total Area Centr	oid
36X36 9.00 SF	
35 MPH 30X30 6.25 SF	

	Size	Area	Total Area	Centroid
	30X30	6.25 SF		
X XXX FEET	24X18	3.00 SF		
	Size	Area	Total Area	Centroid
	36X36	9.00 SF	14.00 SF	3.06 Ft.
X XXX FEET	30X24	5.00 SF		

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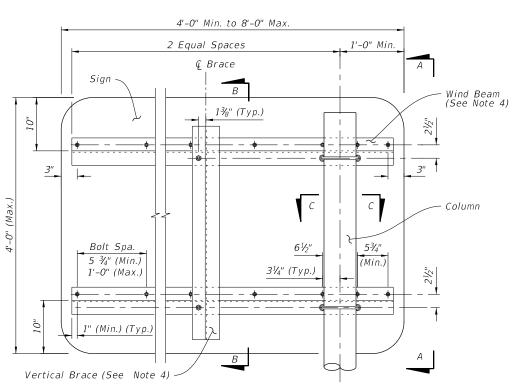
FY 2019-20 STANDARD PLANS



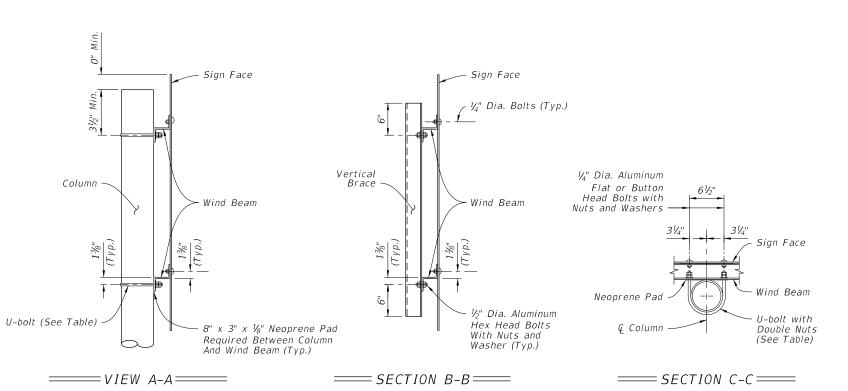
### **GENERAL NOTES:**

- 1. Refer to Index 700-010 for additional notes, assembly of base connection and material specifications not given in this Index.
- 2. Aluminum Columns: ASTM B429 Alloy 6061-T6.
- 3. Place galvanized steel shims between the Sleeve and Post to obtain a tight fit between the Post and Sleeve.
- 4. Wind Beam and Vertical Brace: Aluminum Z 3 x  $2^{1}V_{16}$  x 3.38. Install Vertical Brace on 7'-0" to 8'-0" signs only.
- 5. Provide 2 ~ 0.0149" Thick (28 gauge) and 2 ~ 0.0329" Thick (21 gauge) Brass Shims Per Post. Used brass shims to plumb the post.
- 6. Use nylon washers under the button bolt heads to protect sign sheeting. Use aluminum washers under nut.

COLUMN SELECTION AND FOOTING SIZE TABLE									
Sign Size Height x Length	Column Size Diameter x Thickness	Sleeve Size Diameter x Thickness	U-bolt Diameter	Base Bolt Diameter x Length	Torque Ibs./in	Base Plate Thickness	Footing Depth		
4'-0" x 5'-0" 4'-0" x 6'-0"	4 NPS Schedule 80 (4.5" x 0.337")	5 NPS Schedule 120 (5.563" x 0.5")	1/2"	5⁄8" x 31⁄2"	270 ½ 45	1"	6'-0"		
4'-0" x 7'-0" 4'-0" x 8'-0"	5 NPS Schedule 80 (5.563" x 0.375")	6 NPS Schedule 80 (6.625" x 0.432")	5/8"	¾" × 4"	445 ½ 75	11/8"	6'-6" 7'-0"		



= SIGN DETAIL=



11/01/18

DESCRIPTION: **REVISION** 

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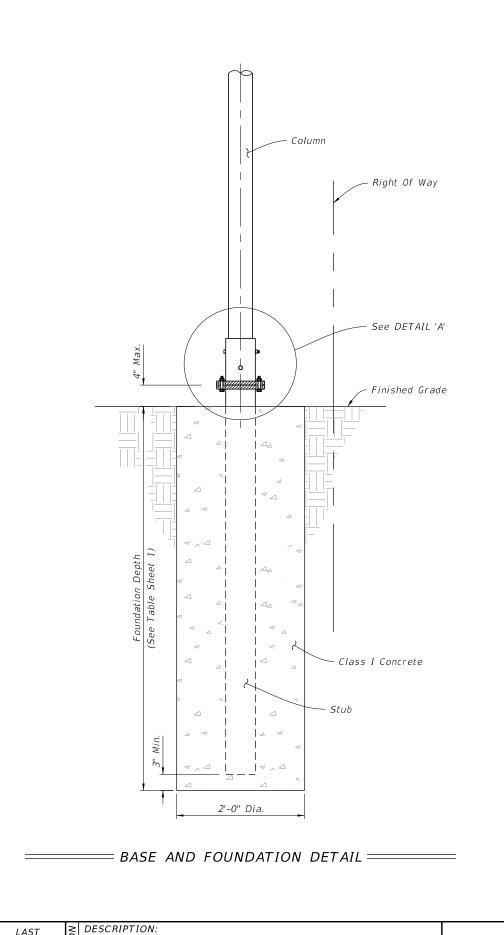
FY 2019-20 STANDARD PLANS

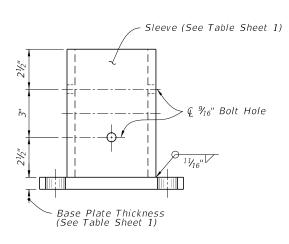
SINGLE COLUMN CANTILEVER GROUND MOUNTED SIGN

INDEX

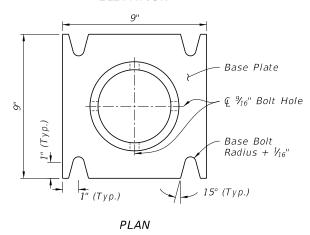
SHEET 1 of 2

700-011

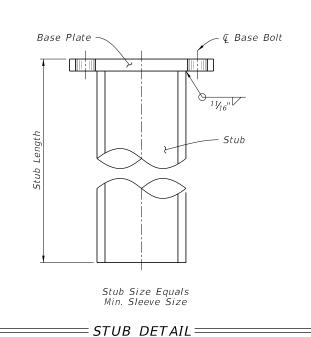


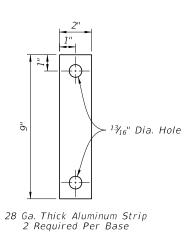


### ELEVATION

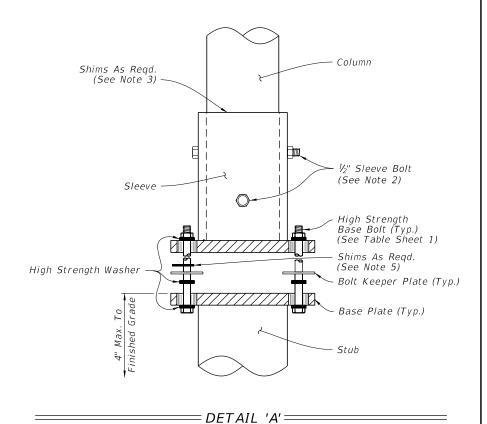


### = SLEEVE & BASE PLATE DETAILS =





### =BOLT KEEPER PLATE DETAIL ===



FDOT

FY 2019-20 STANDARD PLANS SINGLE COLUMN CANTILEVER GROUND MOUNTED SIGN

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SHEET

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- 1. Work with Index 700-010.
- 2. Shop Drawings: Not required.

### 3. <u>Materials:</u>

- A. Steel Plate: ASTM A36 or ASTM A709 Grade 36
- B. Steel Pipe (Support Post): ASTM A501 Schedule 40
- C. Aluminum Pipe: ASTM B429 Alloy 6061-T6
- D. Galvanized U-Bolts, Nuts and Plate Washer
- a. U-Bolts: ASTM A449
- b. Hex Nuts: ASTM A 563 Lock Nuts
- c. Plate Washer: ASTM A 36 or ASTM A709 Grade 36 or 50
- E. Galvanized Anchor bolts, Nuts and Washers:
- a. Anchor Rod: ASTM F1554 Grade 55 fully threaded (for Adhesive Anchors)
- b. Anchor Bolts: ASTM F1554 Grade 55 Grade A Hex
- c. Nuts: ASTM A563 Heavy Hex Locking
- d. Washers: ASTM F436
- F. Adhesive Anchor Bonding Material: Specification 931 Type HV Adhesive.
- G. Weld Material: E70XX
- H. Snap-In Post Cap: UV and weather-resistant glass-filled polyester cap

#### 4. Coating:

- A. U-Bolts, Threaded Rods, Nuts and Washers: ASTM F2329
- B. Other Steel: ASTM A123

### 5. <u>Fabrication:</u>

- A. Weld: Specification 460-6.4
- B. Hot dip galvanize after fabrication

#### 6. Construction:

- A. Locate Sign Support a minimum of 5 feet from an open joint or transition (sign stationing may be adjusted to accommodate this requirement.
- B. Base plate must be flush with back of Traffic Railing
- C. Anchors in Traffic Railings:
- a. Install Adhesive Anchors in accordance with Specification 416 except perform field test on one anchor per sign support location.
- b. Use templates and tie anchors as necessary to maintain correct placement of C-I-P Embedded Anchors c. Do not drill into existing conduit
- D. Temporary Signs on Permanent Traffic Railings: Same as Permanent except Field testing of anchors is not required

### 7. Removal of Temporary Signs on Permanent Traffic Railings:

- A. Cut anchor rods flush with the top of the traffic railing
- B. Coat anchors with Type F-1 epoxy to prevent corrosion
  - a. Extend coating 2 inches beyond edge of cut anchor rods
- b. Epoxy coating 1/16" thick minimum

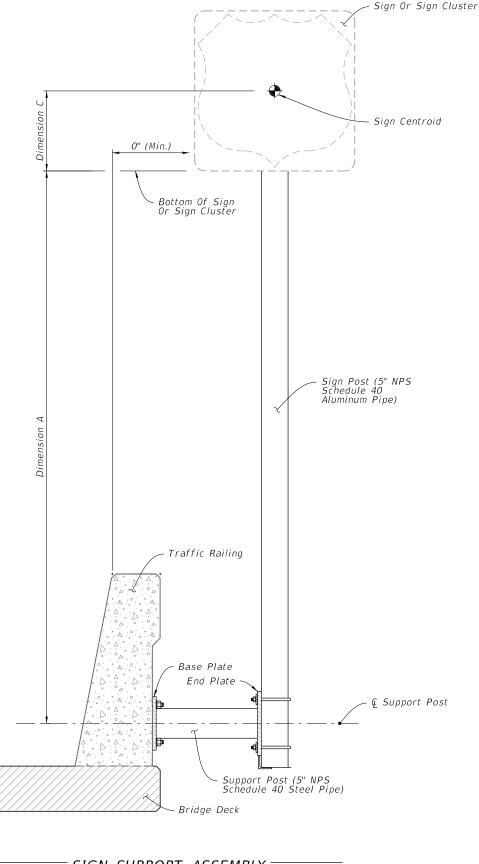
### 8. <u>Payment:</u>

Include the cost of all materials and labor in the cost of the single post sign assembly.

SIGN LIMITATIONS TABLE						
MAX. SIGN AREA (SF)	MAX. SIGN CENTROID HEIGHT (DIM. A + DIM. C)					
25	9'-7"					

Dimension A = Distance from centerline of the Support Post to the bottom of the sign or sign cluster.

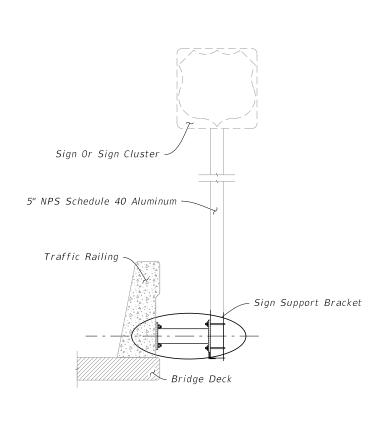
Dimension  $C = Vertical \ distance \ from \ the \ bottom \ of \ the \ sign \ or \ sign \ cluster$  to the Centroid of the sign or sign cluster.

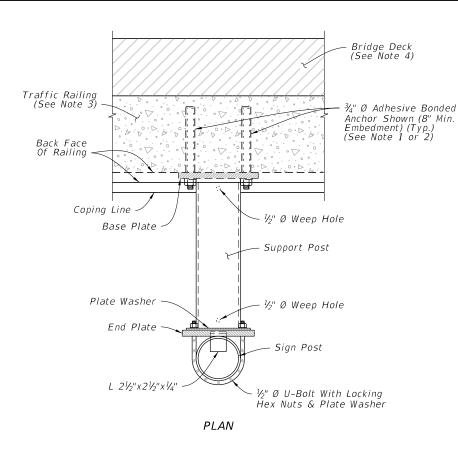


==== SIGN SUPPORT ASSEMBLY ======

LAST DESCRIPTION:
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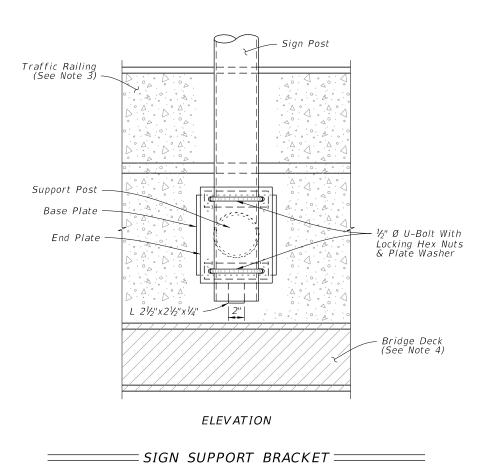
### 1. Existing Traffic Railings:

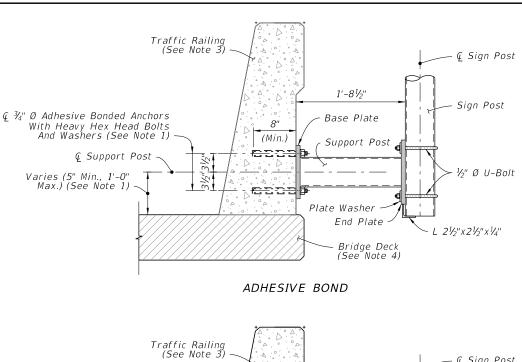
- A. Locate existing conduit prior to drilling and adjust placement of base plate as necessary to avoid damaging existing conduit. Base plate must be flush with back of traffic railing. Maintain a minimum cover 2" from face of traffic railing to tip of Adhesive Anchor.
- B. For concrete parapets less than 10" thick, through bolt ¾" Ø Heavy Hex Head Bolts with Nuts and Washers in lieu of Adhesive Bonded Anchors. Bolt heads shall not protrude more than  $1\frac{1}{2}$ " beyond traffic face of railing.
- C. For through bolting, countersink the nut and washer so that the bolt and nut does not extend beyond the face of the traffic railing. Do not exceed a countersink depth and diameter of  $2\frac{1}{2}$ ".

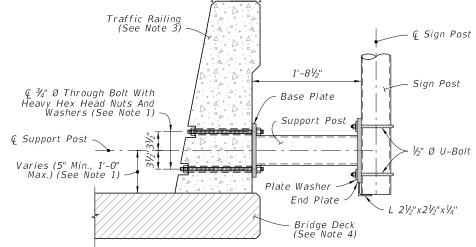
### 2. New Traffic Railings:

A. Optional Couplers are shown for slipforming; keep Anchor Bolt coupler threads free of concrete.

- 3. 36" Single-Slope Traffic Railing shown, other Traffic Railings and Parapets are similar.
- 4. Bridge Deck shown, Approach Slab and Retaining Wall are similar

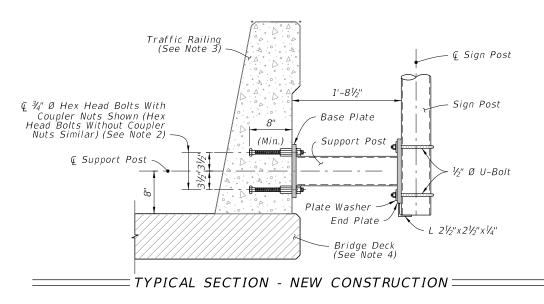


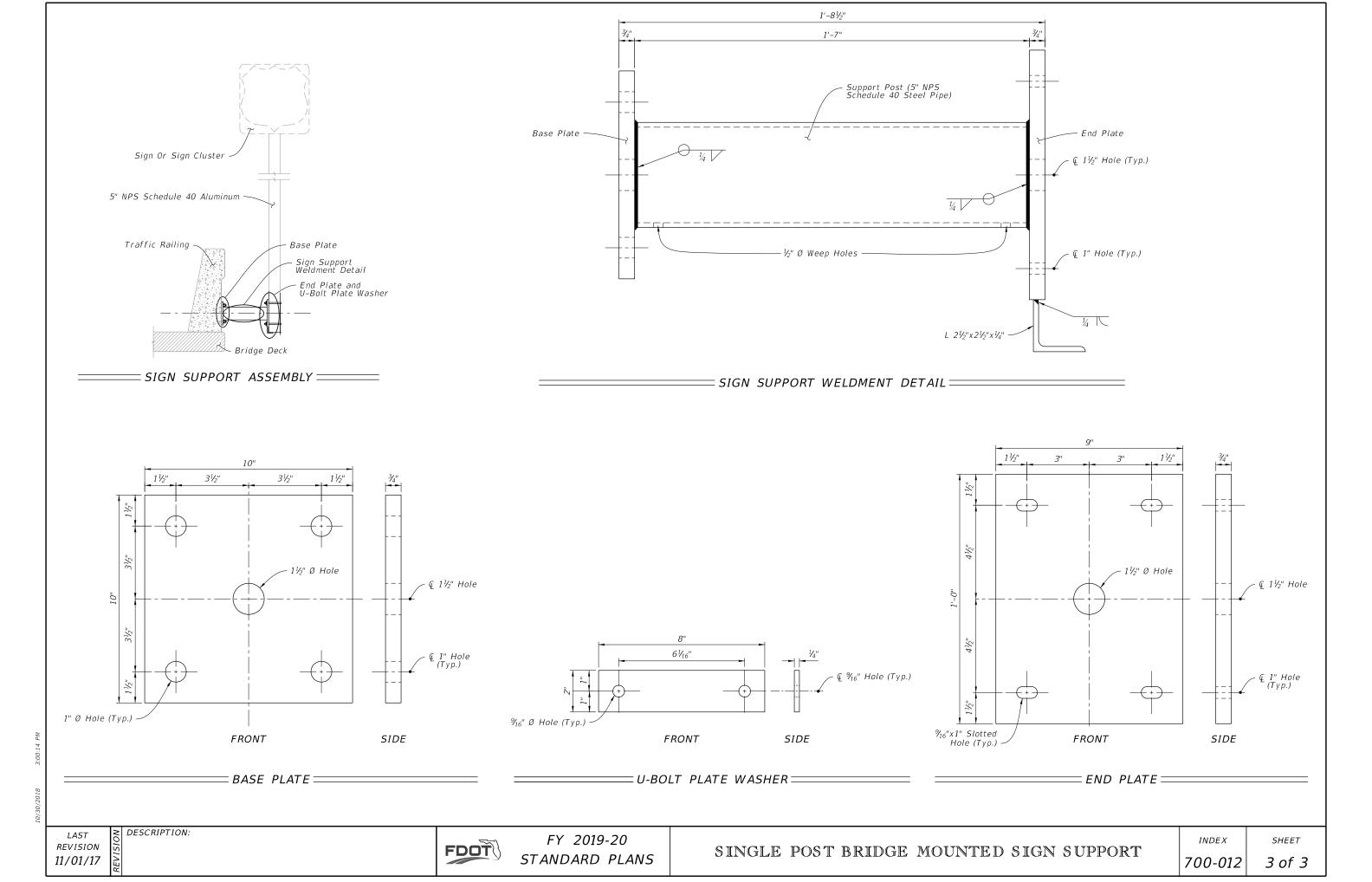




THROUGH BOLTING

### TYPICAL SECTION - EXISTING RAILING

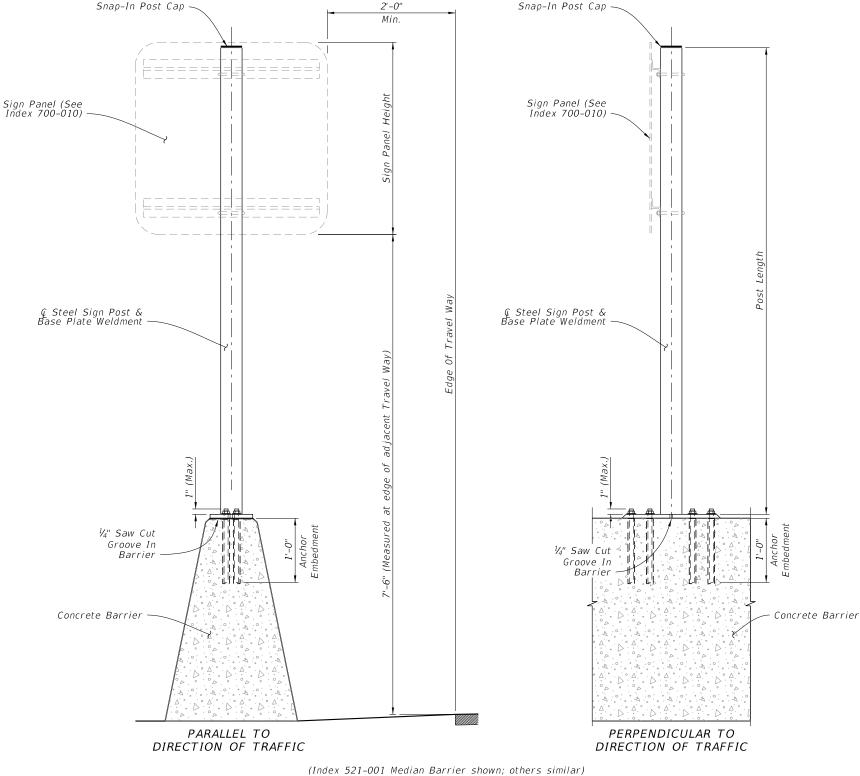




- 1. Work with Index 700-010.
- 2. Shop Drawings: Not required.
- 3. Materials:
- A. Steel Plate: ASTM A36 or ASTM A709 Grade 36 B. Steel Pipe (Support Post): ASTM A53 Grade B Schedule 40
- C. Galvanized U-Bolts, Nuts and Plate Washer
- a. U-Bolts: ASTM A449
- b. Hex Nuts: ASTM A 563 Lock Nuts
- c. Plate Washer: ASTM A 36 or ASTM A709 Grade 36 or 50
- D. Galvanized Anchor Bolts, Nuts and Washers:
- a. Anchor Rod: ASTM F1554 Grade 55 fully threaded (for Adhesive Anchors)
- b. Anchor Bolts: ASTM F1554 Grade 55 Grade A Hex
- c. Nuts: ASTM A563 Heavy Hex Locking
- d. Washers: ASTM F436
- E. Adhesive Anchor Bonding Material: Specification 937 Type HV Adhesive
- F. Weld Material: E70XX
- G. Snap-In Post Cap: UV and weather-resistant glass-filled polyester cap
- A. U-Bolts, Threaded Rods, Nuts and Washers: ASTM F2329
- B. Other Steel: ASTM A123
- 5. Fabrication:
- A. Weld: Specification 460-6.4 B. Hot dip galvanize after fabrication
- 6. Construction:
- A. Locate Sign Support a minimum of 5 feet from an open joint or transition (sign stationing may be adjusted to accommodate this requirement B. Base plate must be flush with top of Railing
- C. Anchors in Traffic Railings:
- a. Install Adhesive Anchors in accordance with Specification 416 except perform field test on one anchor per sign support location
  b. Use template and tie anchors as necessary to maintain correct placement of C-I-P
- Embedded Anchors
- c. Do not drill into existing reinforcing
  D. Temporary Signs on Permanent Traffic Railings, Same as Permanent except field testing of anchors is not required
- E. Temporary Signs on Temporary Railings/Barriers:
  - a. Install Sign Supports at the midpoint along the length of a single segment
  - b. Avoid drilling through existing reinforcement; use of metal detector not required.
- c. Field testing of anchors is not required
- 7. Removal of Temporary Signs on Permanent Traffic Railings:
- A. Cut anchor rods flush with the top of the railing
- B. Coat anchors with Type F-1 epoxy to prevent corrosion a. Extend coating 2 inches beyond edge of cut anchor rods
  - b. Epoxy coating 1/16"thick minimum

Include the cost of all materials and labor in the cost of the single post sign assembly.

TABLE 1 - SIGN PANEL AND POST SIZING						
Max. Sign Area (SF) Post (NPS)						
Temporary Signs	≤ 24	3.0				
Permanent Signs	< 13.5	3.0				
Permanent Signs	13.5 < Sign < 20	3.5				



= ELEVATION =

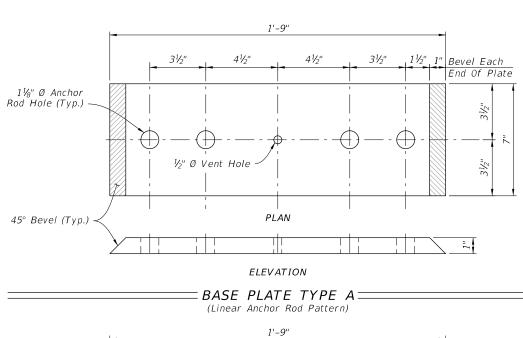
DESCRIPTION: **REVISION** 11/01/18

FY 2019-20 STANDARD PLANS

SINGLE POST MEDIAN BARRIER MOUNTED SIGN SUPPORT

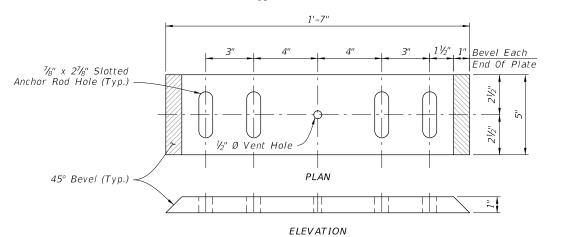
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SHEET 1 of 2



# 1½" <u>1</u>" Bevel Each End Of Plate 1⅓" Ø Anchor Rod Hole (Typ.) ½" Ø Vent Hole PLAN 45° Bevel (Typ.)



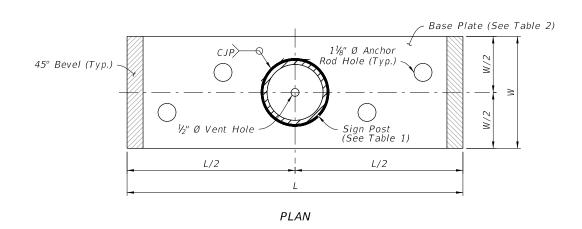


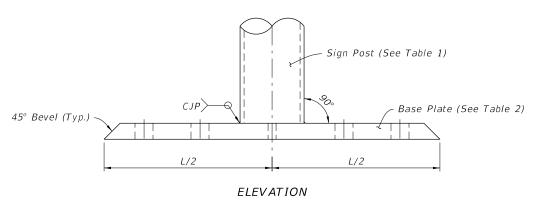
BASE PLATE TYPE C

### NOTES:

- 1. Place anchor rods in a staggered or linear pattern as necessary to avoid reinforcing.
- 2. Use a staggered pattern for all temporary barriers.

TABLE 2 - BASE PLATE TYPE AND ANCHOR ROD SIZING						
Index Type/Application Base Plate Type Anchor Rod (						
521-001	Full Wall	В	1"			
521-001	Cantilever or L-Wall	Α	$I^{-}$			
All listed above Plus 102-110 & 102-100	Temporary Signs	С	3/4"			





 $\equiv$  SIGN SUPPORT WELDMENT DETAIL  $\equiv$ 

(Staggered Anchor Rod Pattern shown)

REVISION 11/01/17

DESCRIPTION:

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FY 2019-20 STANDARD PLANS

SINGLE POST MEDIAN BARRIER

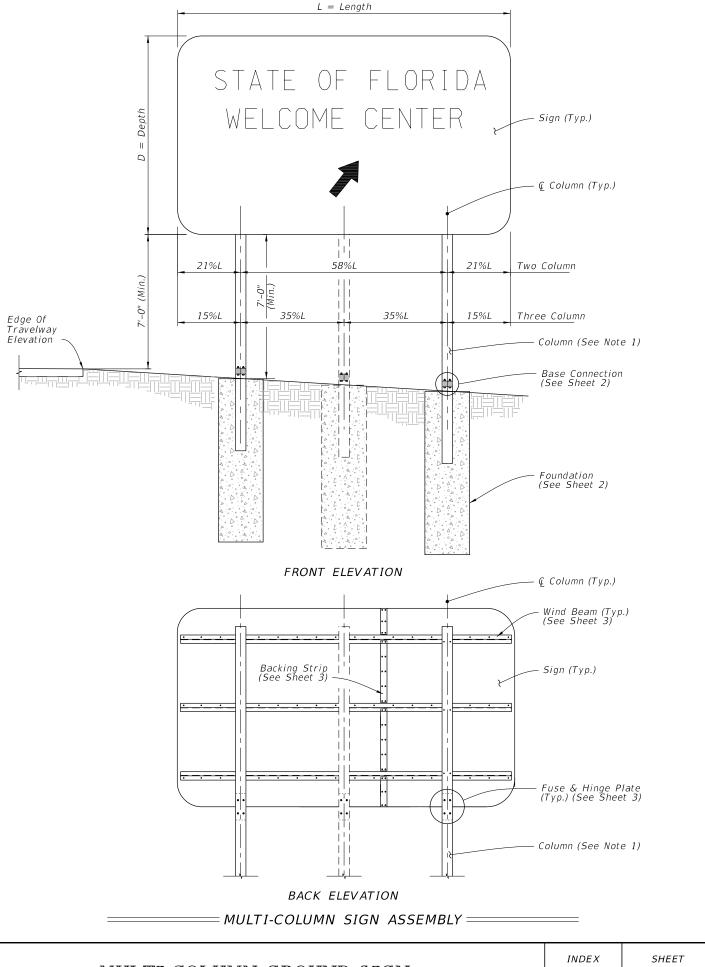
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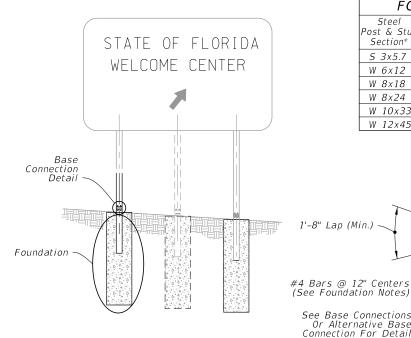
SHEET

700-013 2 of 2

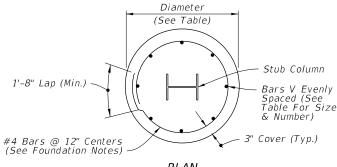
### GENERAL NOTES:

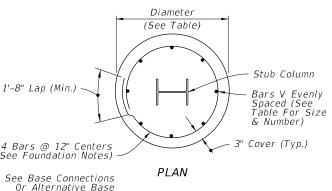
- 1. Verify Column lengths in the field prior to fabrication.
- 2. Shop drawings:
- A. Sign Support Shop drawings are not required when fabricated in accordance with this Index and support columns do not exceed the length shown in the plans by more than 2'-0".
- B. Sign Panels: Horizontal panel splices are allowed at interior wind beams for sign panels with a depth ("D") greater than 10 feet. Shop drawings required for horizontal panel splice details.
- C. When shop drawings are required, obtain approval prior to fabrication.
- 3. Materials:
- A. Sign Panel Mounting Materials:
  - a. Aluminum Bars, and Extruded Shapes: ASTM B221, Alloy 6061-T6 or Alloy 6351-T5
  - b. Aluminum Structural Shapes: ASTM B308, Alloy 6061-T6
- B. Sign Support Structure Materials:
  - a. Steel Plates and Structural Shapes: ASTM A36 or ASTM A709, Grade 36
- b. Steel Weld Metal: E70XX
- c. Shims: Brass ASTM B36 or Galvanized Steel
- C. Aluminum Bolts, Nuts and Washers:
  - a. Flat Head and Button Head Bolts: ASTM F 468, Alloy 2024-T4
  - b. Hex Nuts: ASTM F467, 2024-T4
- c. Washers: ASTM B221, Alloy 2024-T4
- D. Stainless Steel Bolts, Nuts and Washers Alloy Group 2, Condition A, may be substituted for the Aluminum bolts as follows:
  - a. Bolts: ASTM F593, CW1 or SH1
  - b. Nuts: ASTM F594,
- E. High Strength (H.S.) Steel Bolts, Nuts and Washers:
  - a. Galvanized Hex Head Bolts: ASTM F3125, Grade A325, Type 1
  - b. Galvanized Nuts: ASTM A563 Hex, Grade DH
  - c. Galvanized Washers: ASTM F436
- F. Concrete: Class I.
- G. Reinforcing Bars or Welded Wire Reinforcement (WWR): Specification 415
- 4. Coatings:
- A. Aluminum Fasteners: Anodic coating (0.0002 inches min.) and chromate sealed
- B. Galvanize High Strength Steel Bolts Nuts and Washers: ASTM F2329
- C. Galvanize all other steel items (excluding stainless steel): Hot-dip ASTM A123
- D. Treat damaged galvanizing in accordance with Specification 562
- 5. Fabrication:
- A. All Base Connections and Stub Column materials are steel unless otherwise specified.
- B. Drill or sub-punch and ream holes in Fuse Plates and Hinge Plates
- C. Weld Base Plate to Post & Stub or if using the Alternate Connection Detail weld Base Plate and Stiffeners to Post and Stub (Sheet 2)
- D. Hot dip galvanize after fabrication; Remove all drips, runs or beads on base plate within washer contact areas (Including saw cuts)
- 6. Construction:
- A. Install the Sign Structure foundation in accordance with Specification 455. Orient Stub Post according to direction of traffic (Sheet 2)
- B. Tighten all high strength bolts except Base Bolts in accordance with Specification 700.
- C. Assemble Post to Stub with Base Bolts and three flat washers per bolt (See Base Connection Details, Sheet 2). Tighten Base Bolts in accordance with Instructions Notes on Sheet 2.





FO	FOUNDATION DATA							
Steel Post & Stub Section*	Dia.	Depth	Stub Column Length	Reinf. Bars V				
S 3x5.7	2'-0"	4'-0"	3'-0"	10-#6				
W 6x12	2'-0"	6'-0"	3'-0"	10-#6				
W 8x18	2'-4"	7'-6"	4'-0"	8-#8				
W 8x24	2'-4"	8'-6"	4'-0"	8-#8				
W 10x33	2'-4"	10'-3"	4'-0"	8-#8				
W 12x45	2'-8"	11'-3"	5'-0"	10-#8				

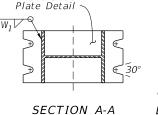




H.S. Base Bolt With 3 Washers & Hex Nut on Each Bolt. See Table for Bolt Dia. & Torque. See Assembly Of Base Instructions. Washer (Typ.) Top Base Plate Remove All Galvanizing Shims As Required Runs Or Beads Base Bolt In Washer Area  $Dia. = L_2$ ∠Bolt Keeper Plate Washer (Typ.) Shims As Required - Bottom Base Plate Washer (Typ.) Foundation

______Direction of Traffic [____

SIDE ELEVATION



SHIM DETAIL

Base Plate

SECTION B-B

BASE PLATE DETAIL

Stiffener Plate

 $W_2$ 

1/4"

1/4"

5/16"

5/16"

W

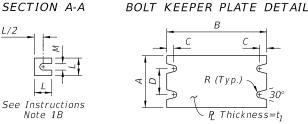
R

(Ibf∗in)

| 270±45 | 5-1/8"

R(Typ.)

Plate Thickness=0.0149" (28 Gauge)



BASE PLATE DETAIL

Depth of

R (Typ.)

Plate Thickness=0.0149"

(28 Gauge)

BOLT KEEPER PLATE DETAIL

STIFFENER PLATE DETAIL

Section

	BASE CONNECTION DATA								SH	'IM	
Steel Post & Stub Section*	А	В	С	D	R	t ₁	L ₂	W ₁	Torque (Ibf*in)	L	М
S 3x5.7	4"	7"	3/4"	2"	5/16"	1"	1/2"	1/4"	90 ± 20	1-1/4"	9/16"
W 6x12	4"	10"	3/4"	2"	3/8"	1-5/8"	5/8"	1/4"	270 ± 45	1-3/8"	11/16"
W 8x18	5-1/4"	12-1/2"	7/8"	2-3/4"	7/16"	1-3/4"	3/4"	3/8"	445 ± 75	1-3/4"	13/16"
W 8x24	6-1/2"	12-1/2"	7/8"	3-1/4"	7/16"	1-3/4"	3/4"	3/8"	445 ± 75	2-1/8"	13/16"
W 10x33	8"	16"	1-1/4"	4-3/4"	9/16"	2"	1"	1/2"	580 ± 90	2-3/8"	1-1/16"
W 12x45	10"	18"	1-1/4"	6"	9/16"	2"	1"	1/2"	580 ± 90	2-3/4"	1-1/16"

### st Designations: (Nominal Depth in inches) x (weight in pounds per linear foot).

FRONT ELEVATION

### FOUNDATION NOTES:

The Contractor may use Welded Wire Reinforcement (WWR) for foundation reinforcing.

== MULTI-COLUMN SIGN ASSEMBLY ==

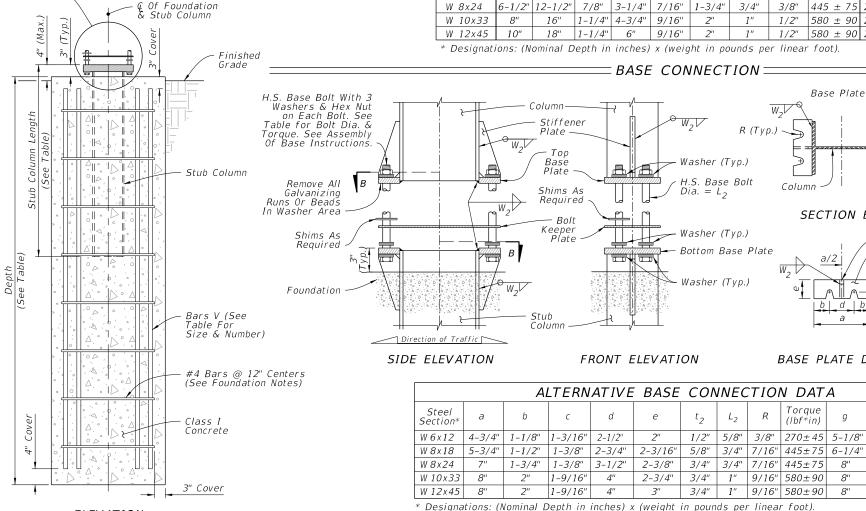
At the Contractors option, the #4 tie bars at 12" o.c. may be replaced by D10 Spiral Wire @ 6" pitch, with three flat turns at the top and one flat turn at the bottom in accordance with Specification 415.

### **INSTRUCTIONS NOTES:**

- 1. Assembly of Base Instructions.
- A. Place one washer on each Base Bolt between the Bottom Base Plate and the head of high strength Base Bolt; place the next washer between the Bottom Base Plate and the Bolt Keeper Plate; add the Top Base Plate section and place the third washer between the Top Base Plate
- B. Shim as required to plumb column. Provide 2-0.0149" thick (28 gauge) and 2-0.0329" thick (21 gauge) shims per column.
- 2. H.S. Base Bolt L₂ Tightening Instructions:
- A. Tighten Base Bolts to the maximum possible with a 12" to 15" wrench (this will bed the washers and shims and clear the bolt threads).
- B. Loosen each Base Bolt one turn.

DESCRIPTION:

- C. Under the supervision of the Engineer, use a calibrated wrench to tighten bolts to the torque prescribed in the Table. Over tightened Base Bolts will not be permitted.
- D. Burr threads at junction with nut to prevent nut loosening. Treat damaged galvanizing.



ALTERNATIVE BASE CONNECTION =

FOUNDATION AND BASE CONNECTION DETAILS

2-3/4"

**REVISION** 11/01/18

FDOT

ELEVATION

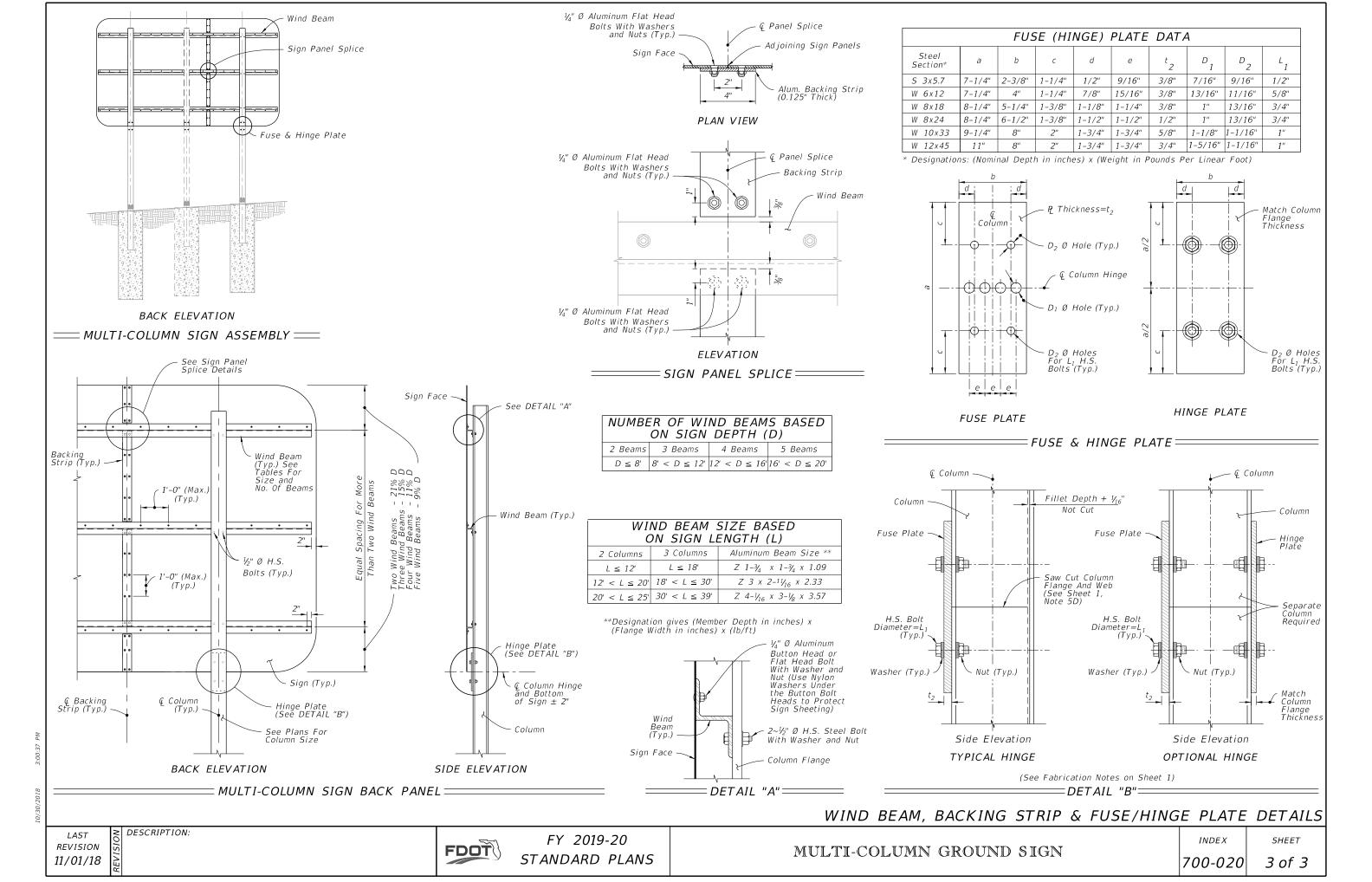
**FOUNDATION** 

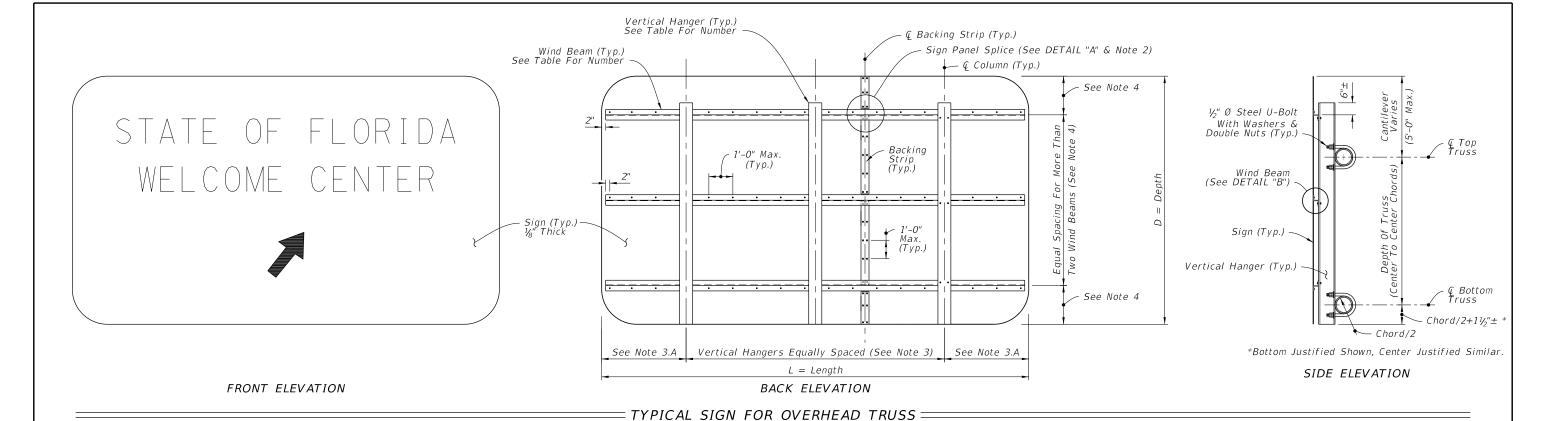
FY 2019-20 STANDARD PLANS

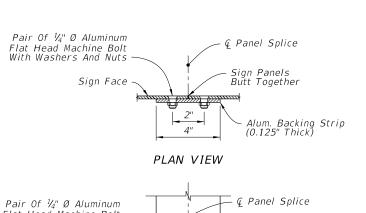
MULTI-COLUMN GROUND SIGN

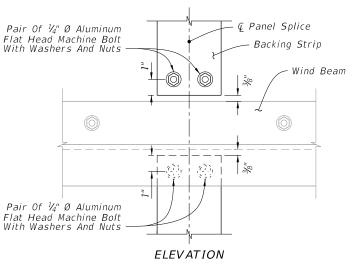
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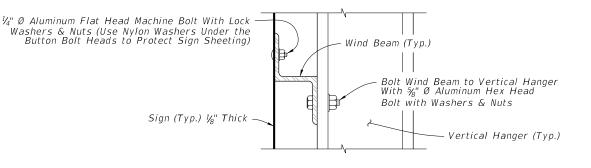
SIGN PANEL SPLICE

DETAIL "A"=

WIN	D BEAM T	TABLE (Z	3 x 2 ¹¹ / ₁₆	x 2.33)
Number	of Horizontai	Wind Beams	Based on Sig	n Depth (D)
2 Beams	3 Beams	4 Beams	5 Beams	6 Beams
D ≤ 5'	5' < D ≤ 9'	9' < D ≤ 12'	$12' < D \le 15'$	$15' < D \le 18'$

HAN	HANGER TABLE (I 6 $\times$ 4.69 or $Z$ 5 $\times$ 3 $\frac{1}{4}$ $\times$ 6.19)							
Number of	Number of Vertical Hanger Beams Based on Wind Speed and Sign Length (L)							
	2 Hangers	3 Hangers	4 Hangers	5 Hangers	6 Hangers			
130 mph	L ≤ 20'	20′ < L ≤ 30′	$30' < L \le 40'$	$40' < L \le 50'$				
150 mph	L ≤ 18′	18' < L ≤ 27'	27' < L ≤ 35'	$35' < L \le 45'$	$45' < L \le 50'$			
170 mph	L ≤ 15'	$15' < L \le 20'$	20′ < L ≤ 28′	28' < L ≤ 35'	35' < L ≤ 43'			

NOTE: For Monroe County designs, use 170 mph values but with  $Z = 5 \times 3 - \frac{1}{4} \times 6.19$  vertical hanger beams only.



DETAIL "B"

### GENERAL NOTES

- 1. Work this Index with Index 700-040 and 700-041.
- 2. The number and location of the Panel Splices are determined by the Sign
- 3. Spacing of Vertical Hangers:
- A. Two Vertical Hanger = 21.0% Three Vertical Hanger = 15.0% L Four Vertical Hanger = 11.0% L Five Vertical Hanger = 9.0% L Six Vertical Hanger = 7.0% L
- B. Spacing of vertical hangers may be varied slightly as necessary to clear the truss struts and diagonals at panel points
- 4. Spacing of Wind Beams:

Two Wind Beams = 21.0% D Three Wind Beams = 15.0% D Four Wind Beams = 11.0% D Five Wind Beams = 9.0% D Six Wind Beams = 7.0% D

- 5. Shop Drawings:
- A. Required for Sign Panels deeper than 10'-0" with a horizontal panel splice. B. Splice must be located in between interior Zee Supports and only allowed on signs greater than 10'-0".
- 6. Materials:
- A. Aluminum.
- a. Bars, and Extruded Shapes: ASTM B 221, Alloy 6061-T6 or Alloy 6351-T5
- b. Structural Shapes: ASTM B308, Alloy 6061-T6
  c. Flat Head and Hex Head Machine Bolts: ASTM F468, Alloy 2024-T4
  d. Hex Nuts: ASTM F467, Alloy 6061-T6 or Alloy 6262-T9
  e. Washers: ASTM B221, Alclad 2024-T4

- a. U-Bolts: ASTM A449 or ASTM A193 B7 b. Nuts: ASTM A563, 2 per leg c. Washers: ASTM F436, (Flat Washers)
- 7. Coatings:
  - A. Aluminum Bolts, Nuts and Washers: Anodic
  - (0.0002 inches min) and chromate sealed
- B. Galvanized Steel Bolts, Nuts and Washers: ASTM F2329
- 8. Wind Speed by county: see Index 715-010.

**REVISION** 11/01/18

DESCRIPTION:

FDOT

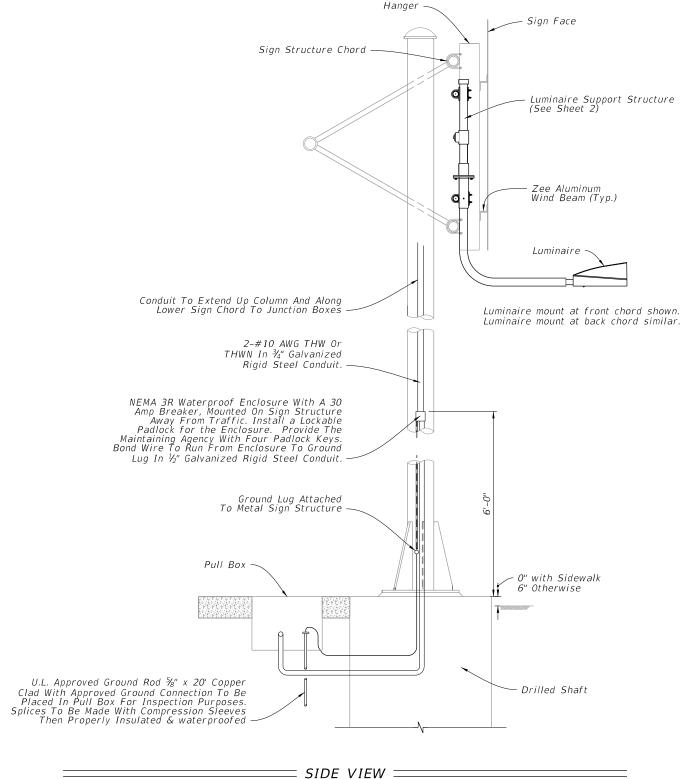
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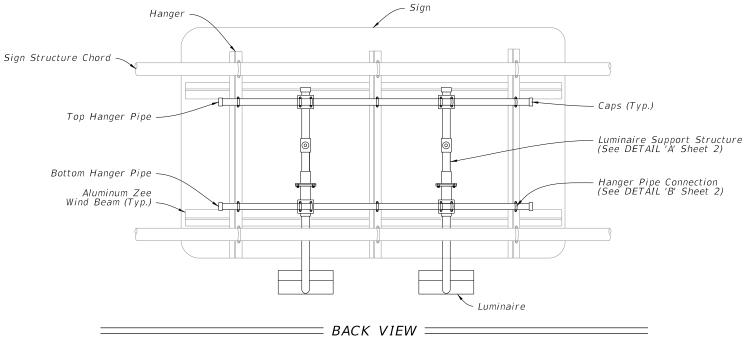
WIND AND HANGER BEAMS FOR OVERHEAD SIGNS

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### PLACEMENT OF SIGN LIGHTS

- 1. This Index details a bottom luminaire support structure. For signs requiring top luminaire support structures, the detail can be reversed.
- 2. Luminaire spacing and arm length is shown on Guide Sign Worksheet.
- 3. The Guide Sign Worksheet indicates the sign luminaire used for basis of design. The contractor may propose a different luminaire by submitting photometric calculations for each lighted sign for review by the Engineer.

### SIGN LIGHTING INSTALLATION

### Roadway Lighting included in contract:

- 1. Power for the sign lighting provided from the roadway lighting circuit.
- 2. Indicate sign location and a pull box location for connection to the sign lights in the lighting plans.
- 3. Lighting contractor installs pull box and loop 2' of lighting circuit conductors in the pull box for connection by the signing contractor.
- 4. Signing contractor furnishes and installs the Luminaires, NEMA 3R enclosure, 30 amp breaker, conduit, conductors and all other electrical equipment necessary for connection to the lighting circuit.

### Roadway Lighting not included in contract:

- 1. Signing plans include the pay item numbers to furnish and install conduit, conductors, ground rods, pull boxes and service point equipment.
- 2. Signing plans indicate the location of the service point equipment and circuit runs.
- Signing contractor provides all electrical equipment necessary for connection of the sign lights.

10/30/2018

LAST REVISION 11/01/17

DESCRIPTION:

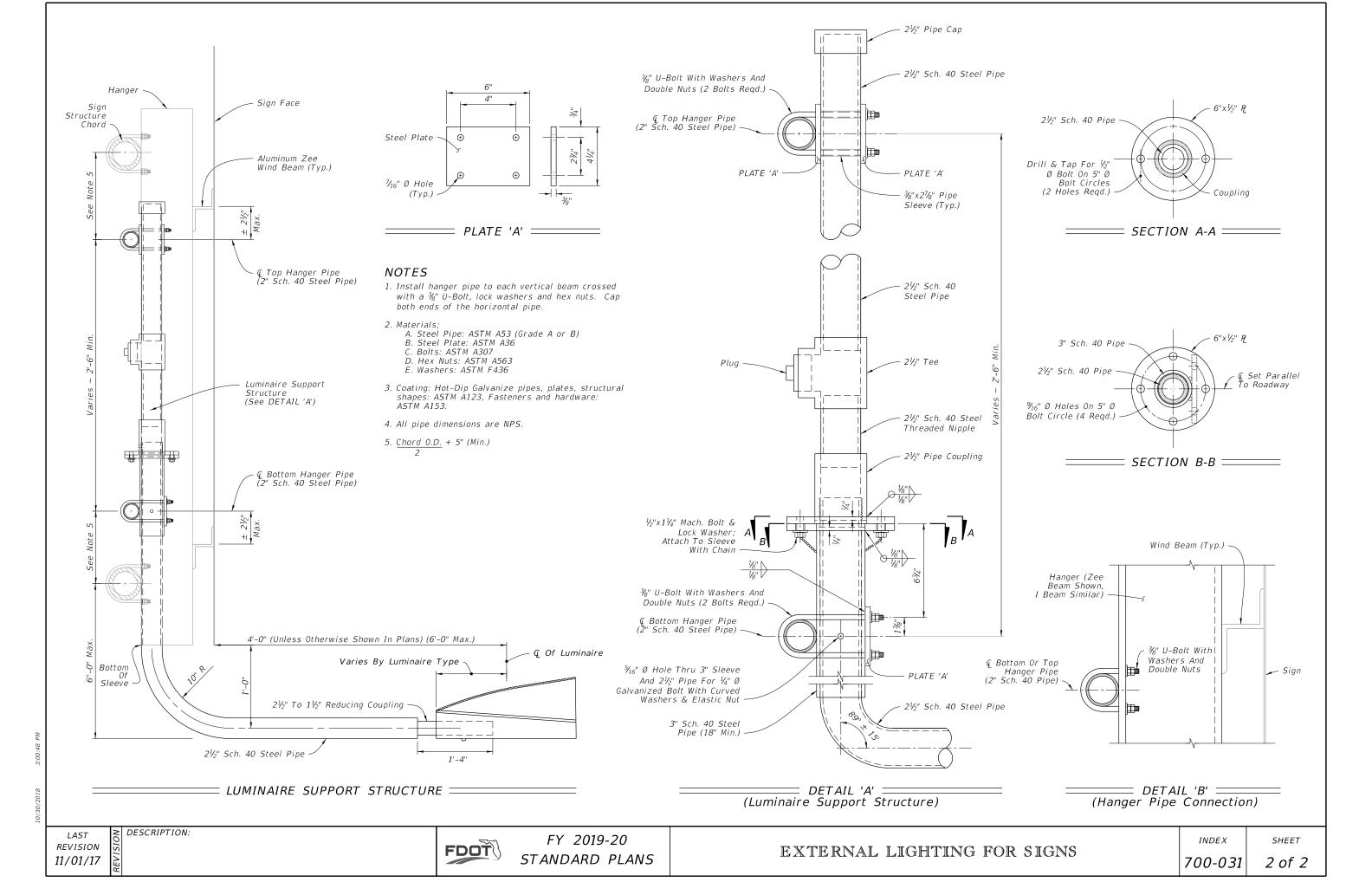


FY 2019-20 STANDARD PLANS

EXTERNAL LIGHTING FOR SIGNS

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SHEET



- 1. Work this Index in conjunction with CANTILEVER SIGN STRUCTURE DATA TABLES in the Plans and Index 700-030.
- 2. Handholes are required at pole base for DMS Structures. Refer to Index 700-090 for Handhole Details.
- 3. Shop Drawings are required.

Obtain Shop Drawing approval prior to fabrication. Include the following: A. Upright Pipe height ('A') and Foundation elevations: Verify dimension in the field prior to submittal to ensure minimum vertical clearances of the sign panel over the roadway.

- B. Height of the foundation above adjacent ground.
- C. Anchor bolt orientation with respect to centerline of truss and the direction of traffic.
- D. Chord Splices
- E. Handholes at pole base (when required).

### 4. Materials:

- A. Sign Structure:
- a. Upright and Chords (Steel Pipe): API 5L X42 PSL2, 42 ksi yield or ASTM A500, Grade B (Min.)
- b. Steel Angles and Structural Plates and Bars: ASTM A709 Grade 36 c. Weld Material: E70XX
- B. Bolts, Nuts and Washers:
- a. High Strength Bolts: ASTM F3125, Grade A325 Type 1 b. Nuts: ASTM A563 Grade DH Heavy-Hex
- c. Washers: ASTM F436 Type 1, one under turned element
- C. Anchor Bolts, Nuts and Washers
- a. Anchor Bolts: ASTM F1554 Grade 55
- b. Nuts: ASTM A563 Grade A Heavy-Hex (5 per bolt)
- c. Plate Washers: ASTM A36 (2 per bolt)
- D. Concrete:
- a. Spread Footing Concrete: Class IV b. Drilled Shaft concrete: Class IV (Drilled Shaft)
- E. Reinforcing Steel: Specification 415

### 5. Fabrication:

- A. Welding: Specification 460-6.4
- B. Chord Splices: "SD" Panel from upright is the closest panel in which a chord splice may be used. See Plans for CANTILEVER SIGN STRUCTURE DATA TABLE. Minimum splice spacing is two truss panel lengths apart.
- C. Upright splices: Not allowed
- D. Structural bolt hole diameters: Bolt diameter plus 1/16"
- E. Anchor bolt hole diameters: Bolt diameter plus 1/5"
- F. Hot Dip Galvanize after fabrication.
- G. Shop assemble the entire structure after galvanizing to validate/document alignment and clearance for bolted connections as well as contact between connecting plates. Take remedial action, if necessary, prior to shipment.
- H. Disassemble, as necessary, and secure components for shipment.

### 6. Coatings:

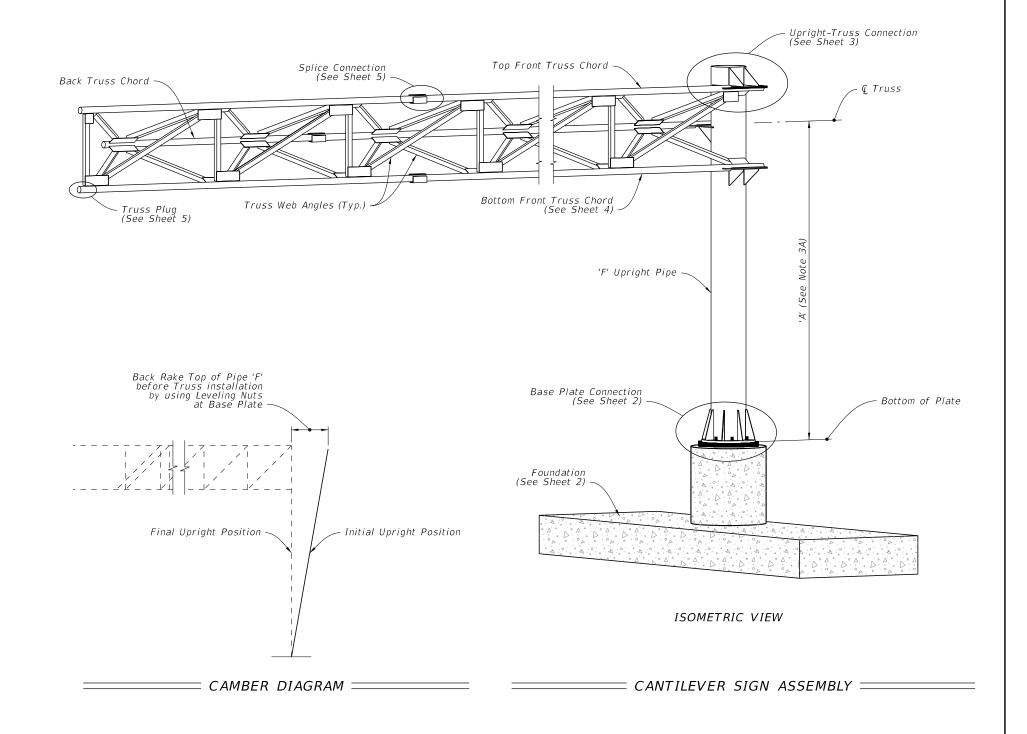
A. Bolts, Nuts and Washers: ASTM F2329

DESCRIPTION:

B. All other steel, including Plate Washers, hot dip galvanize: ASTM A123

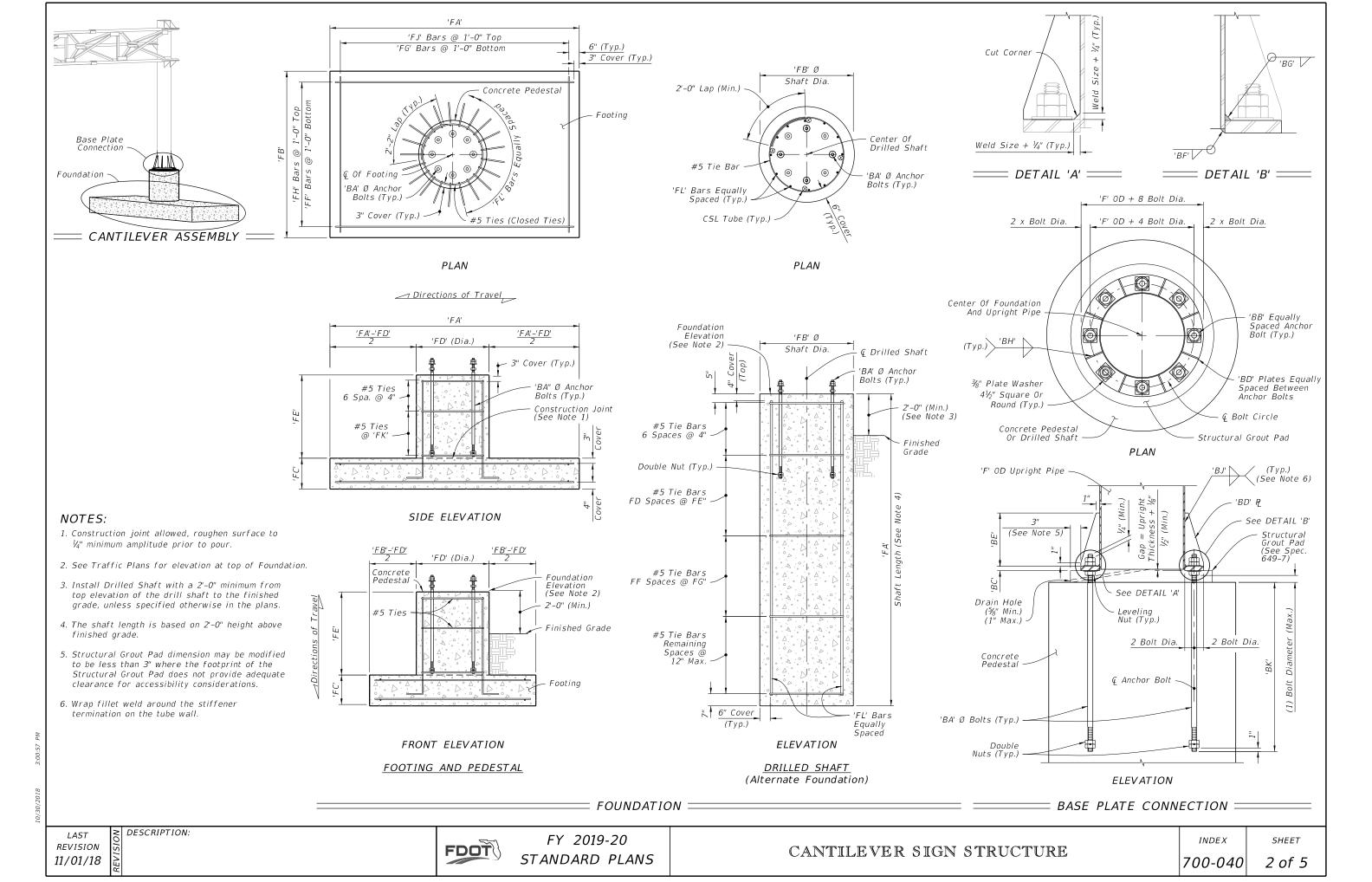
### 7. Construction:

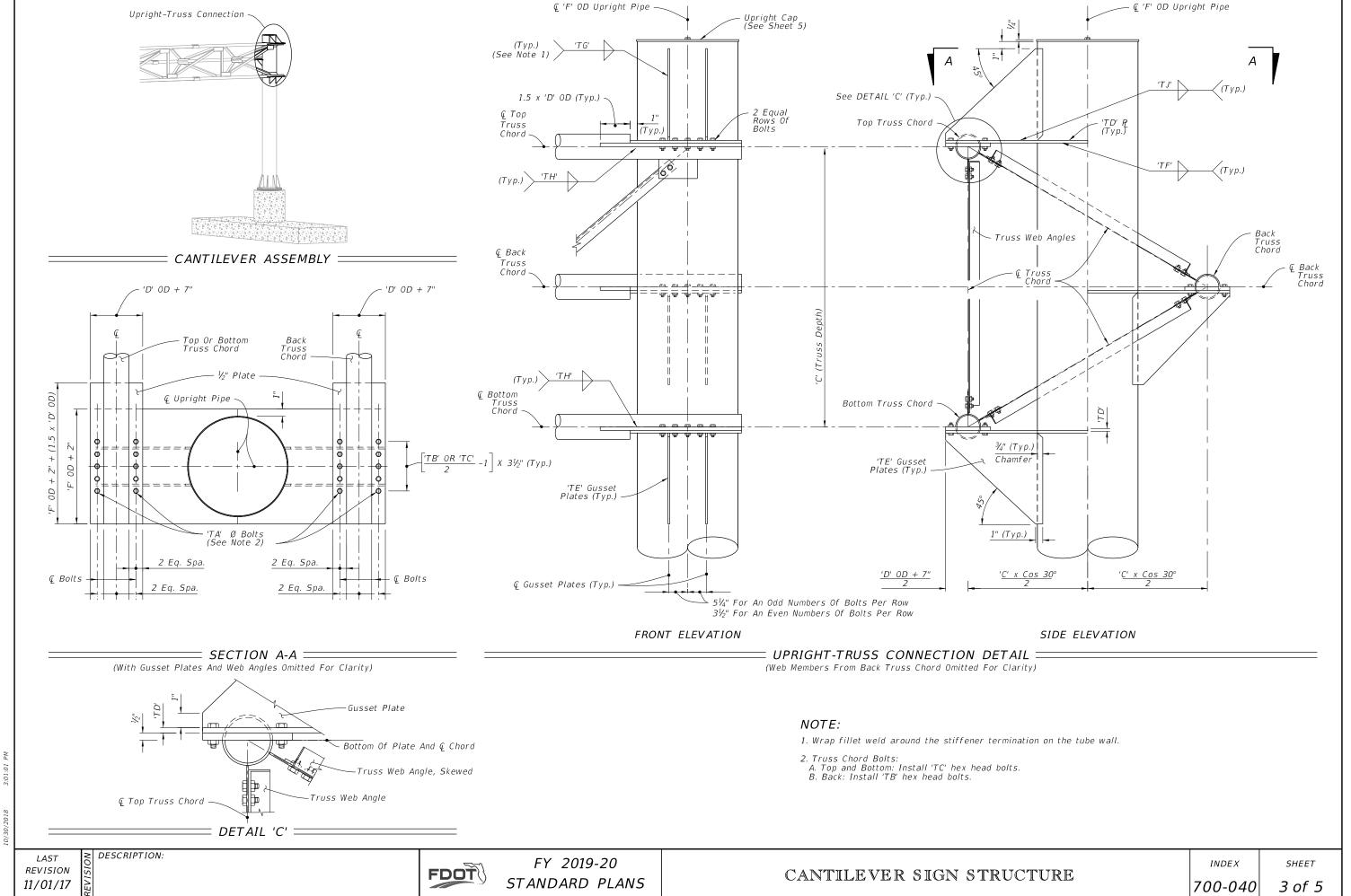
- A. Construct foundation in accordance with Specification 455, except payment is included in the cost of the structure.
- B. Prior to erection, record the as-built anchor locations and submit to
- C. Place backfill above spread footings prior to installation of the sign panels. Do not remove or reduce backfill without prior approval of the Engineer.
- D. Tighten nuts and bolts in accordance with Specification 700. Split-Lock Washers are not permitted.
- E. Install Aluminum Sign Panels as shown in the Plans.
- F. Place structural grout pad with drain between top of foundation and bottom of baseplate in accordance with Specification 649-7.

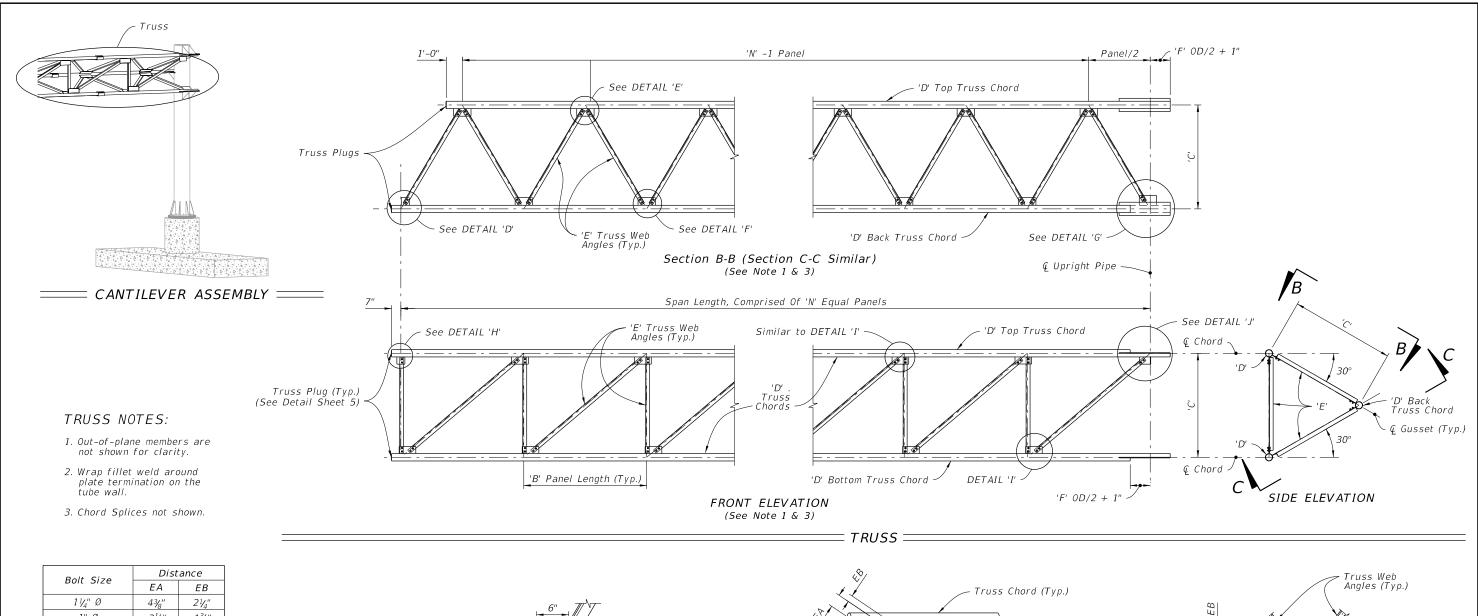


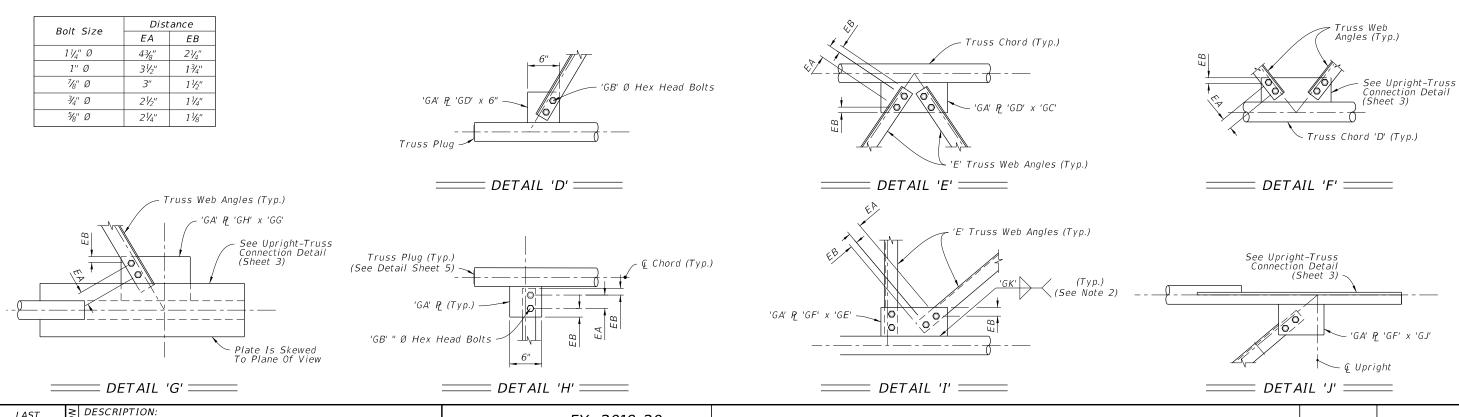
**REVISION** 11/01/17

FDOT









CANTILEVER SIGN STRUCTURE

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FY 2019-20

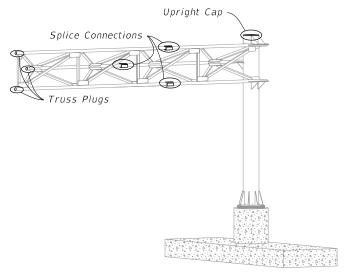
STANDARD PLANS

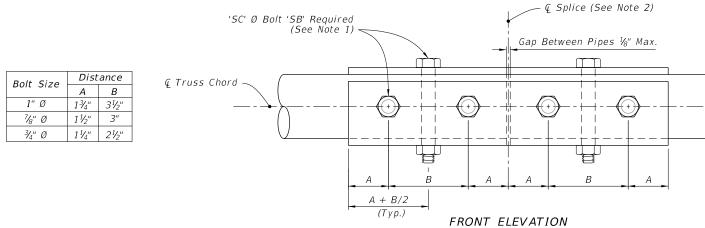
FDOT

LAST

**REVISION** 

11/01/17





'SA' 

© Truss Chord

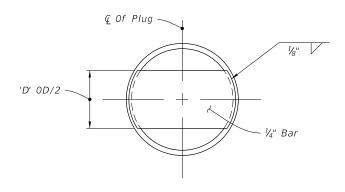
'SC' Ø Bolt (Typ.)

CANTILEVER ASSEMBLY =

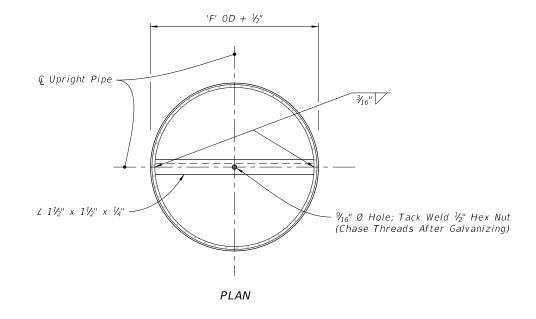
## = SPLICE CONNECTION DETAIL =

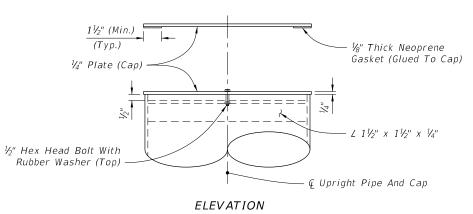
### SPLICE CONNECTION NOTES:

- 1. Only 6 bolts are shown in detail for clarity. (One Half Each Side Of Splice)
- 2. Splices are not permitted for trusses less than or equal to 40', Splice optional for trusses greater than 40'.



= TRUSS PLUG DETAIL ==





 $\equiv$  UPRIGHT CAP DETAIL  $\equiv$ 

LAST REVISION 11/01/17

DESCRIPTION:

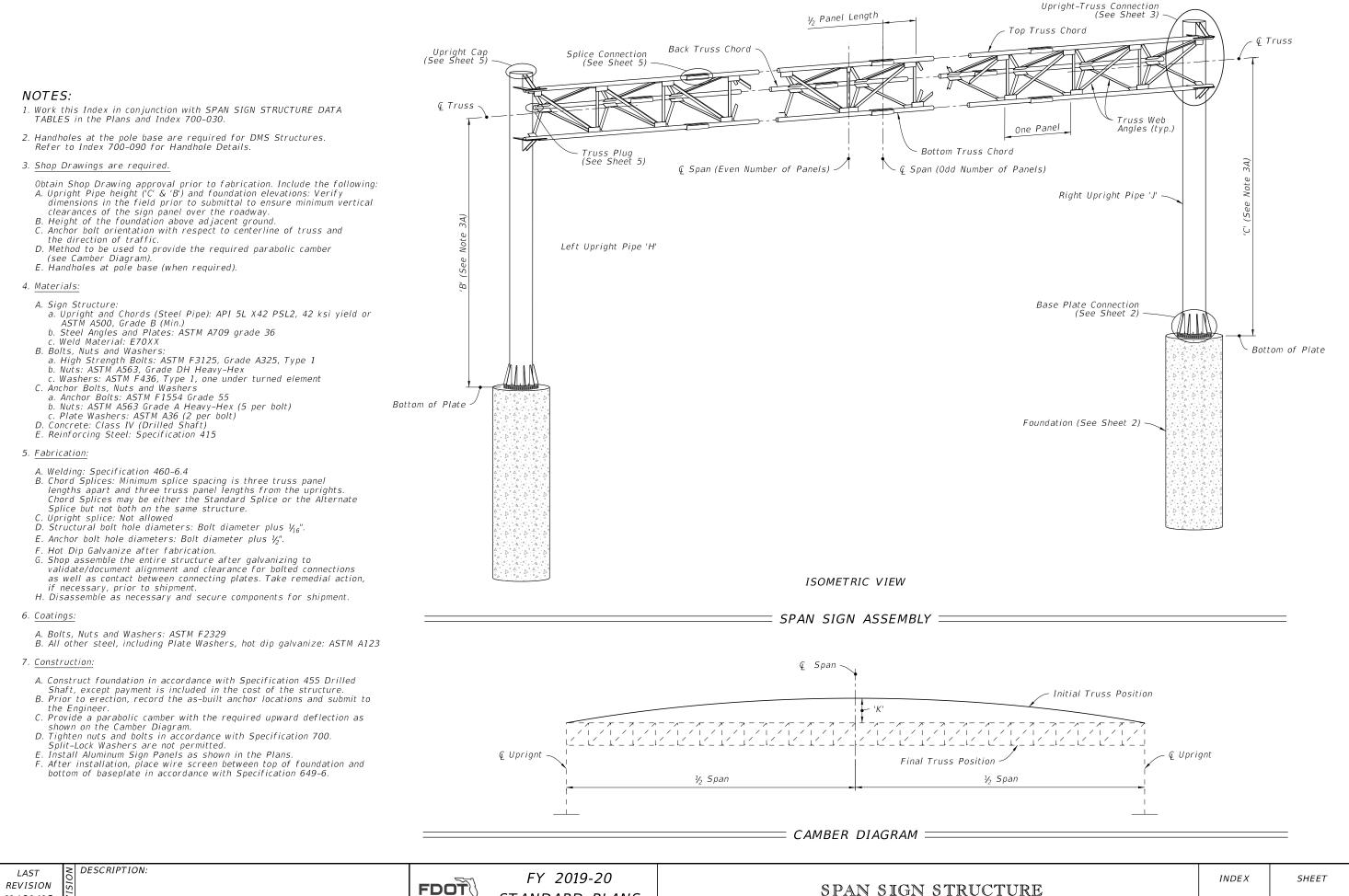
FDOT

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CANTILEVER SIGN STRUCTURE

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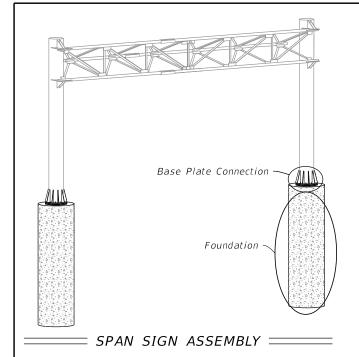
11/01/18

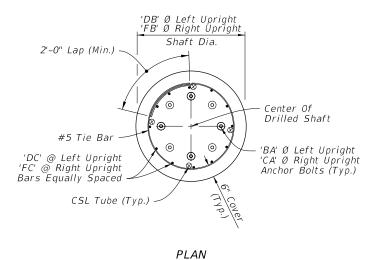
STANDARD PLANS

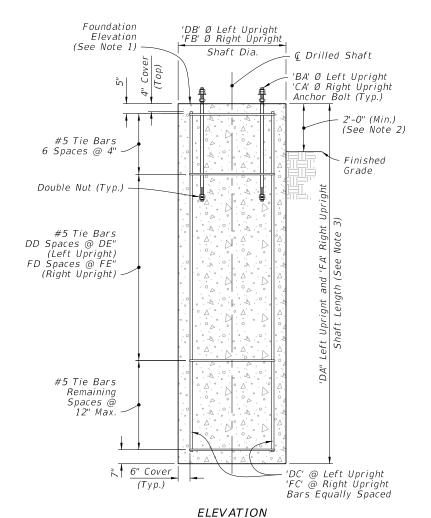
SPAN SIGN STRUCTURE

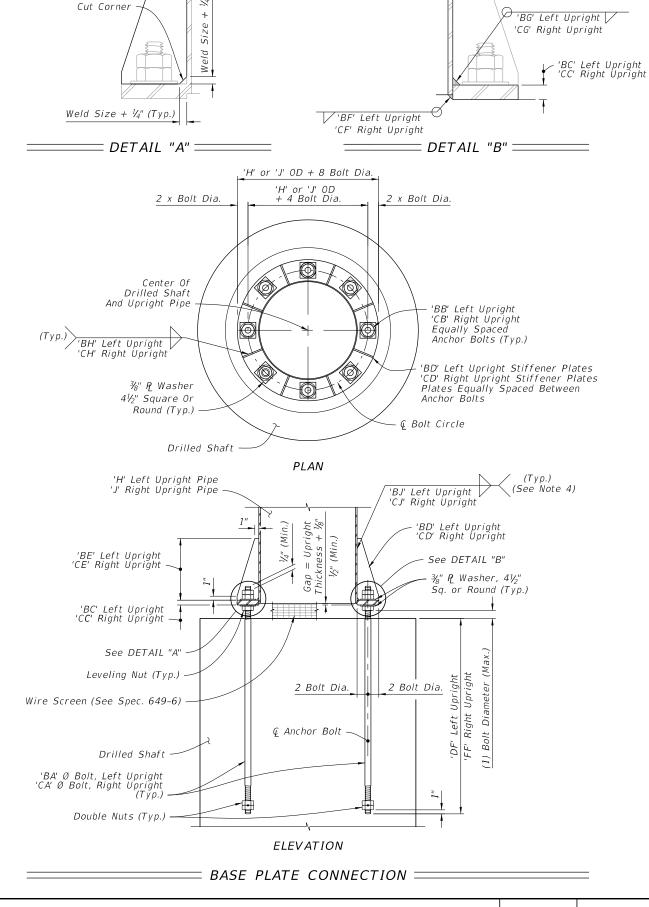
700-041

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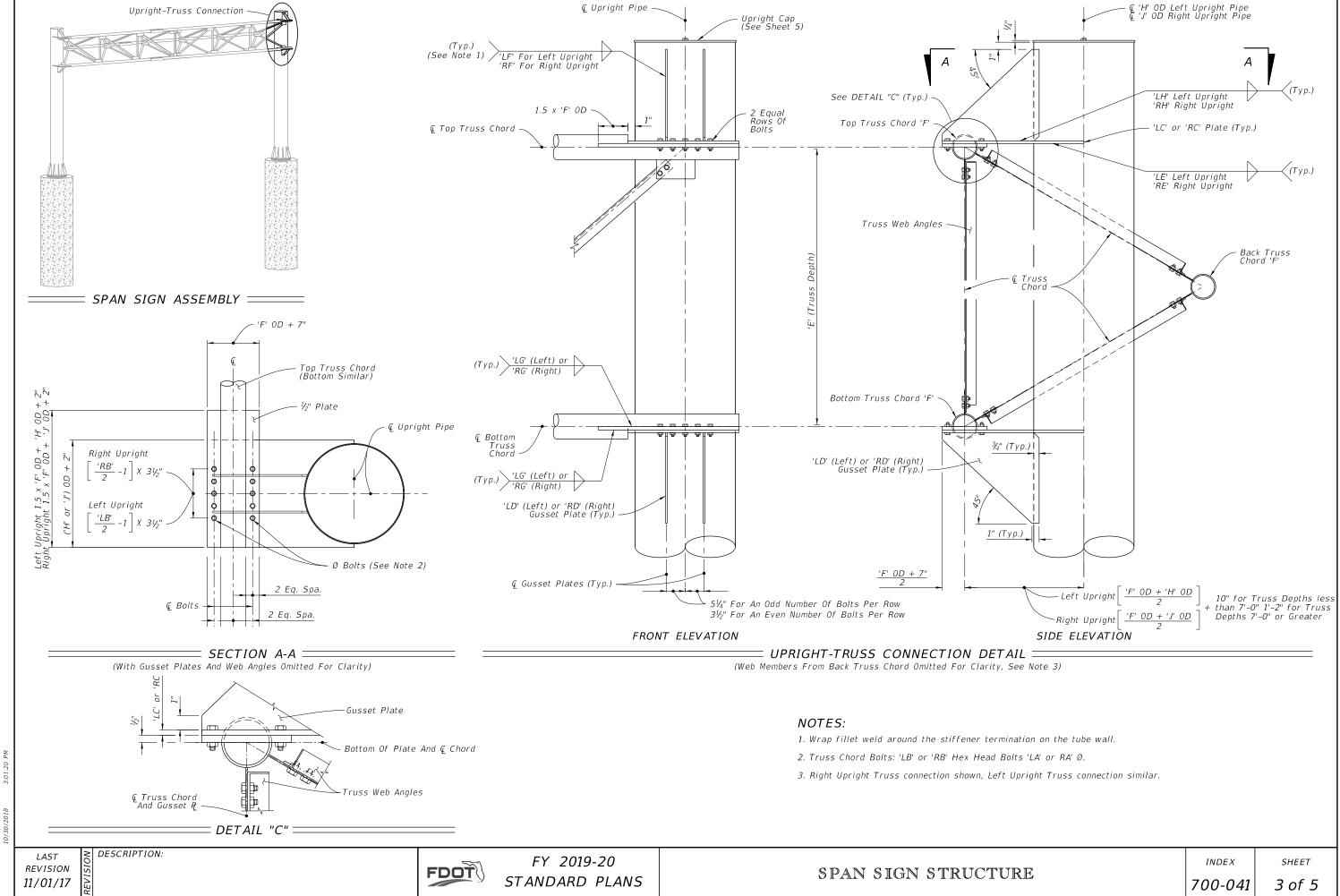
- 1. See Traffic Plans for elevation at top of Foundation.
- 2. Install Drilled Shaft with a 2'-0" minimum from top elevation of the drill shaft to the finished grade, unless specified otherwise in the plans.
- 3. The shaft length is based on 2'-0" height above finished grade.
- 4. Wrap fillet weld around the stiffener termination on the tube wall (Typ).

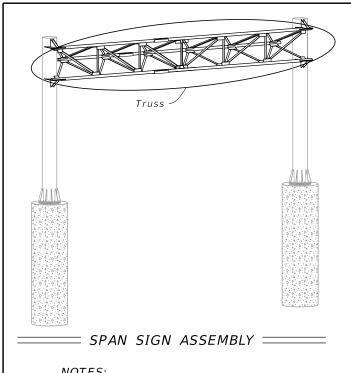
DRILLED SHAFT

FOUNDATION =

700-041

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'GA' P2 'GD' x 'GC

LAST

**REVISION** 

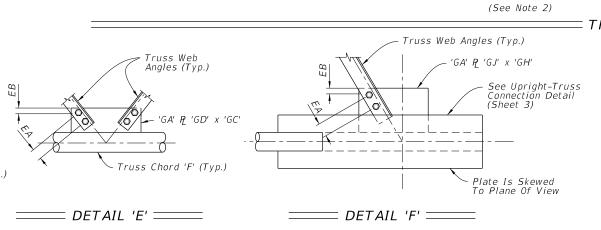
11/01/17

- 1. Out-of-plane members are not shown for clarity.
- 2. Back truss chord and attached angles are not shown for clarity.

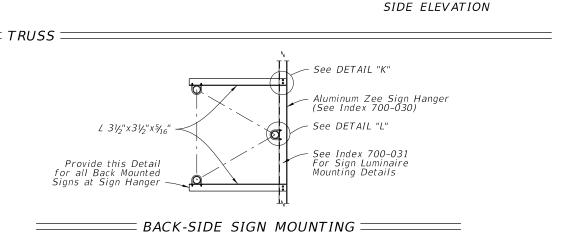
Truss Chord 'F' (Typ.)

3. Wrap fillet weld around plate termination on the tube wall

Bolt Diameter	Distance (in.)				
(in.)	EA	EΒ			
11/4	4¾	21/4"			
1	31/2	13/4			
7/8	3	11/2			
3/4	21/2	11/4			
5/8	21/4	11/8			



'F' OD Bottom Truss Chord



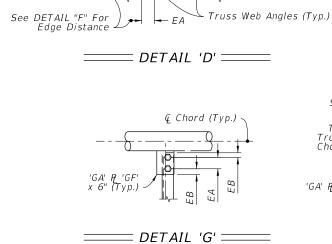
See DETAIL 'F'

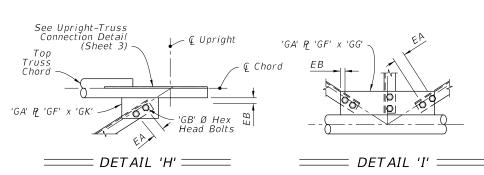
- Ç Right Upright Pipe

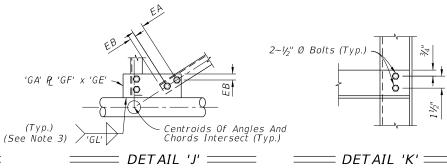
18

← Top Truss Chord

Bottom Truss Chord







'D'-1 Panels

Section B-B (Section C-C Similar) (See Note 1)

Span Length, 'A', Comprised Of 'D' Equal Panels

FRONT ELEVATION

See DETAIL 'G'

'F' OD Back Truss Chord

'G' Truss Web Angles (Typ.)

1/2 The Number of Panels For An Even Number Of Panels

Whole Number For An odd Number Of Panels

See DETAIL 'E'

© Span (Even Number of Panels) -

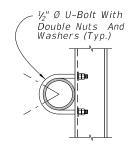
— ← Span (Even Number of Panels)

'F' OD Top Truss Chord

F' OD Top Truss Chord

See DETAIL 'H'

Span (Odd Number of Panels)



DESCRIPTION:

FDOT

See Plug Detail (Sheet 5) (Typ.)

See DETAIL 'D'

← Left Upright Pipe

 $\left[\frac{H' \ OD}{2}\right] + 2$ 

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SPAN SIGN STRUCTURE

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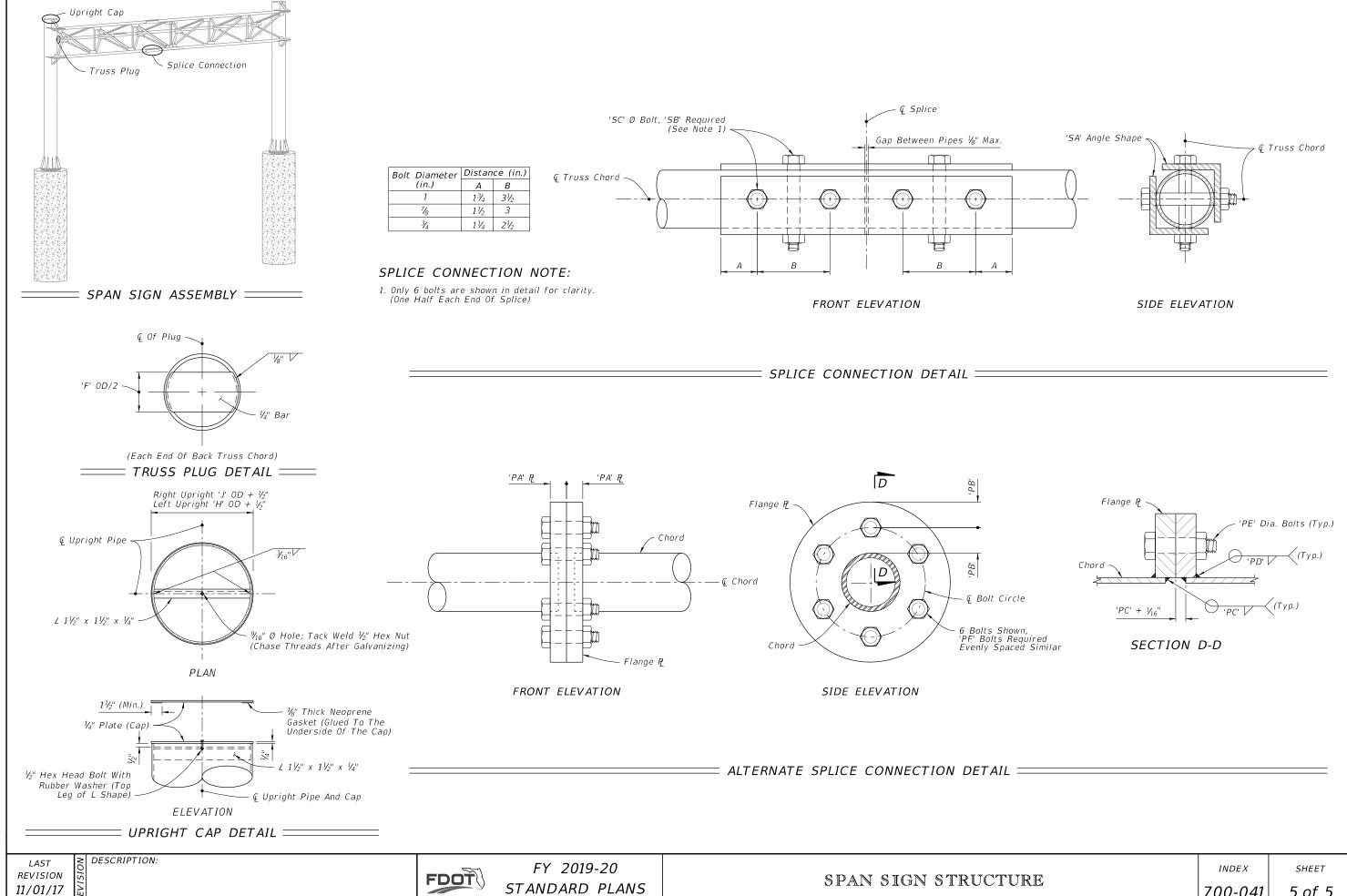
SHEET

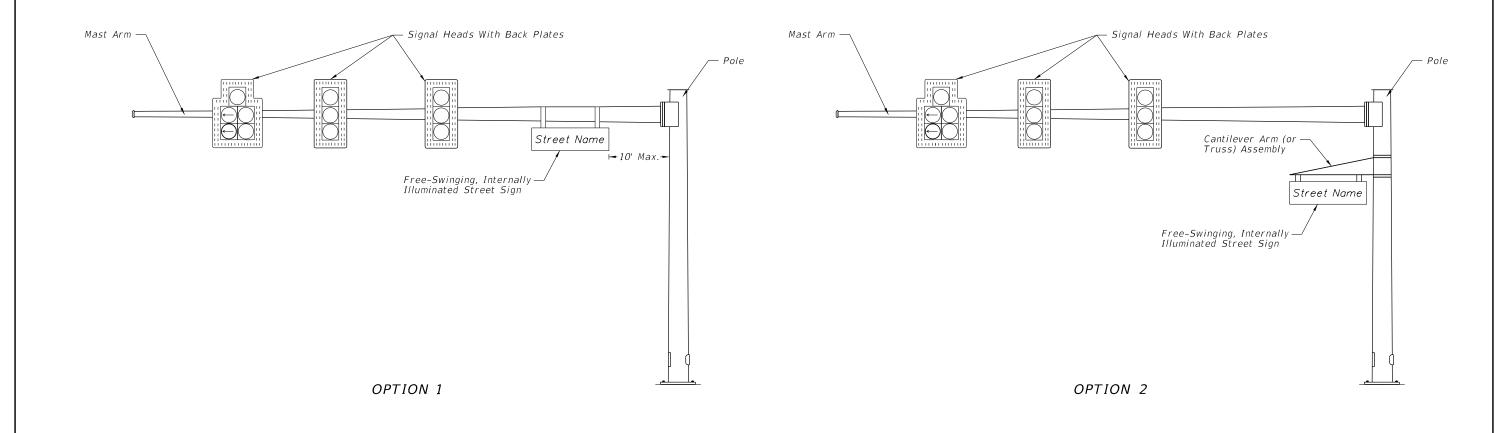
8

Gusset And Back Truss Chord

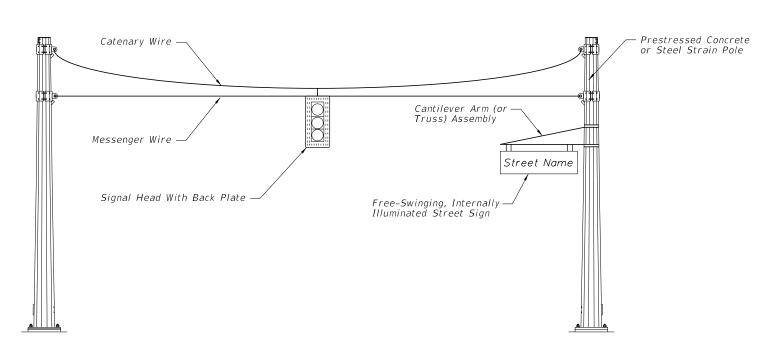
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DETAIL 'L'





### MAST ARM ASSEMBLY



## NOTES:

- 1. Free-swinging, internally-illuminated street signs shall only be installed on the signal pole for span wire assemblies. For mast arm assemblies the street sign may be installed on the
- 2. Free-swinging, internally-illuminated street signs meet the requirements of Specification 700.
- 3. Pole attachments and cantilever arm (or truss) assemblies may be accepted by Contractor certification provided the signs being supported meet the weight and area limitations included in Section 700 for "Acceptance by Certification".
- 4. Pole attachments and cantilever arm (or truss) assemblies supporting signs not meeting the weight or area limitations included in Specification 700 for "Acceptance by Certification" require the submittal of structural calculations and Shop Drawings that have been prepared by and sealed by the Specialty Engineer.

SPAN WIRE ASSEMBLY

**REVISION** 11/01/17

DESCRIPTION:



FY 2019-20 STANDARD PLANS

FREE-SWINGING, INTERNALLY-ILLUMINATED

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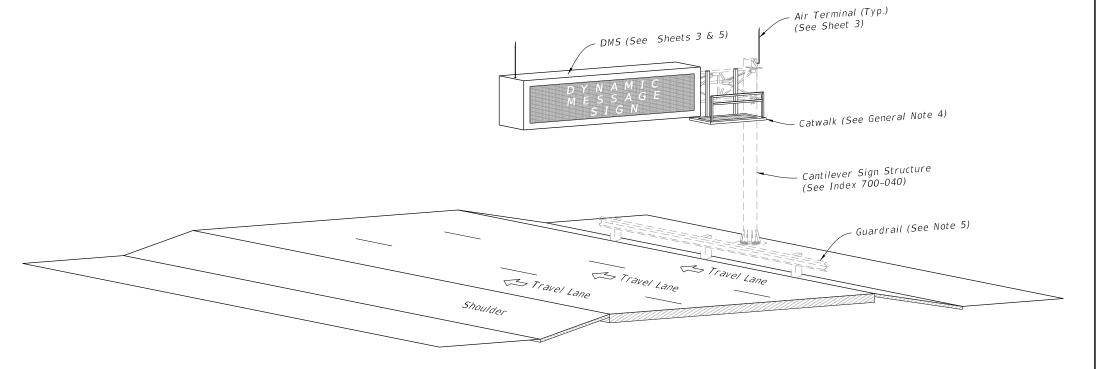
SHEET

700-050 1 of 1

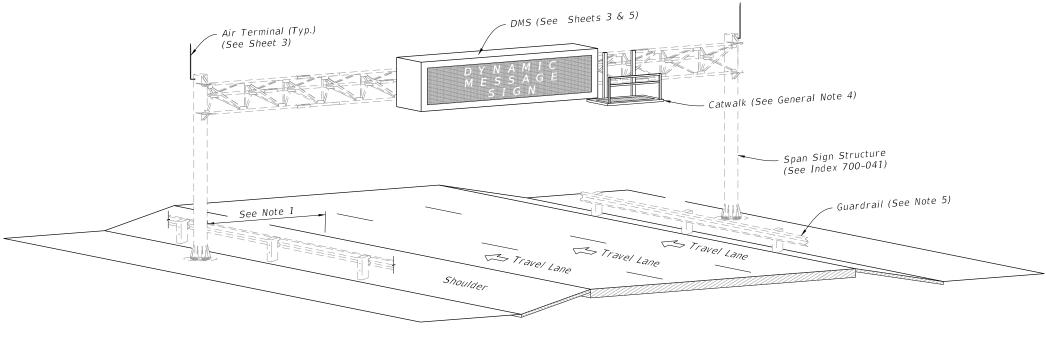
- 2. Furnish and install the Dynamic Message Sign (DMS), sign structure in accordance with Index 700-040 or 700-041. Locate foundations at locations shown in the Plans.
- 3. Shop Drawings are required:
- A. Include the DMS connection
- B. Catwalk design in accordance with AISC, AASHTO, and OSHA requirements, as applicable
- C. Do not start fabrication until the shop drawings are approved
- 4. Extend Catwalk from DMS to outer edge of paved shoulder and not less than 4 feet in length.
- 5. If required, install guardrail at location show in the Plans and in accordance with Index 536-001.
- 6. Materials:
- A. Sign Mounting Components:
- a. Aluminum Structural Shapes: ASTM B308, Alloy 6061-T6
- b. Vertical Hangers: ASTM A704, Grade 36
- c. U-Bolts: ASTM A449 or A193 B7
- d. Steel Bolts, Nuts, and Washers:
- 1. High Strength Bolts: ASTM F3125, Grade A325, Type 1
- 2. Nuts: ASTM F563
- 3. Washers: ASTM F463 (Flat Washer)
- B. Coatings:
- a. All nuts, bolts and washers ASTM F2329
- b. All other steel items ASTM A123
- c. Bolt hole Diameters: Bolt plus  $\frac{1}{16}$ " before galvanizing

### 7. <u>Installation:</u>

- A. See project requirements for location of DMS Cabinet.
- B. Field Adjust pole-mounted DMS cabinet height to achieve best access for maintenance personnel given site condition as directed by the Engineer. Avoid conflicts with stiffeners, handhole and maintenance of anchor bolts.
- C. Locate the sign horizontal on the structure as shown in the Plans. Vertically center the sign enclosure with the centerline of the truss.
- D. Before erection, field drill the bolt holes in the vertical hangers and horizontal mounting member attached to the sign enclosure. Field locate holes to allow vertical hanger placement as shown on the Plans with no conflicts with gusset or splice plates.
- E. Locate threaded couplings on sign side of upright above the sian truss
- F. Connect grounding conductors to the steel framework that has been cleaned to base metal by use of bonding plates having contact area of not less than 8 square inches or by welding or brazing. Drilling and tapping the steel structure to accept a threaded connector is also an acceptable method
- G. If steel framework is to be drilled and tapped to accept threaded connector, the threaded connector shall be galvanized and have at least 5 threads fully engaged and secured with a jam nut to the steel framework.
- H. Bends in the conduit must be greater than the minimum bending radius for the cable contained in the conduit.
- I. Completely encase all data, fiber optic and power cables for the DMS within the sign structure or in conduit.
- J. Permanently stamp/mark foundation to indicate conduit locations.
- K. Transition conduit in foundation to indicate underground conduit with appropriate reducer outside the limits of the foundation.



### CANTILEVER ISOMETRIC VIEW



SPAN ISOMETRIC VIEW

= DYNAMIC MESSAGE SIGN ASSEMBLY =

**REVISION** 11/01/17

DESCRIPTION:



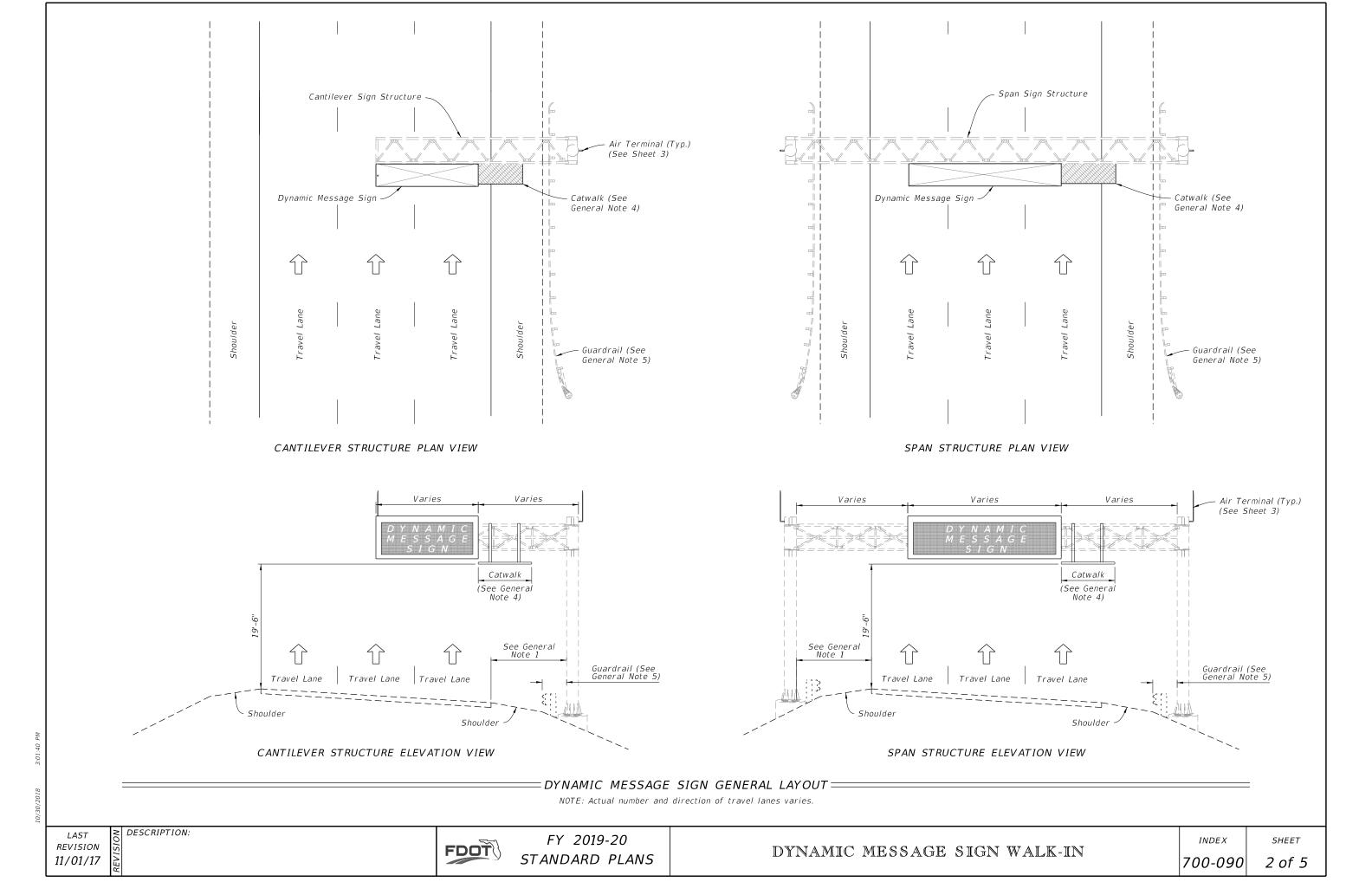
FY 2019-20 STANDARD PLANS

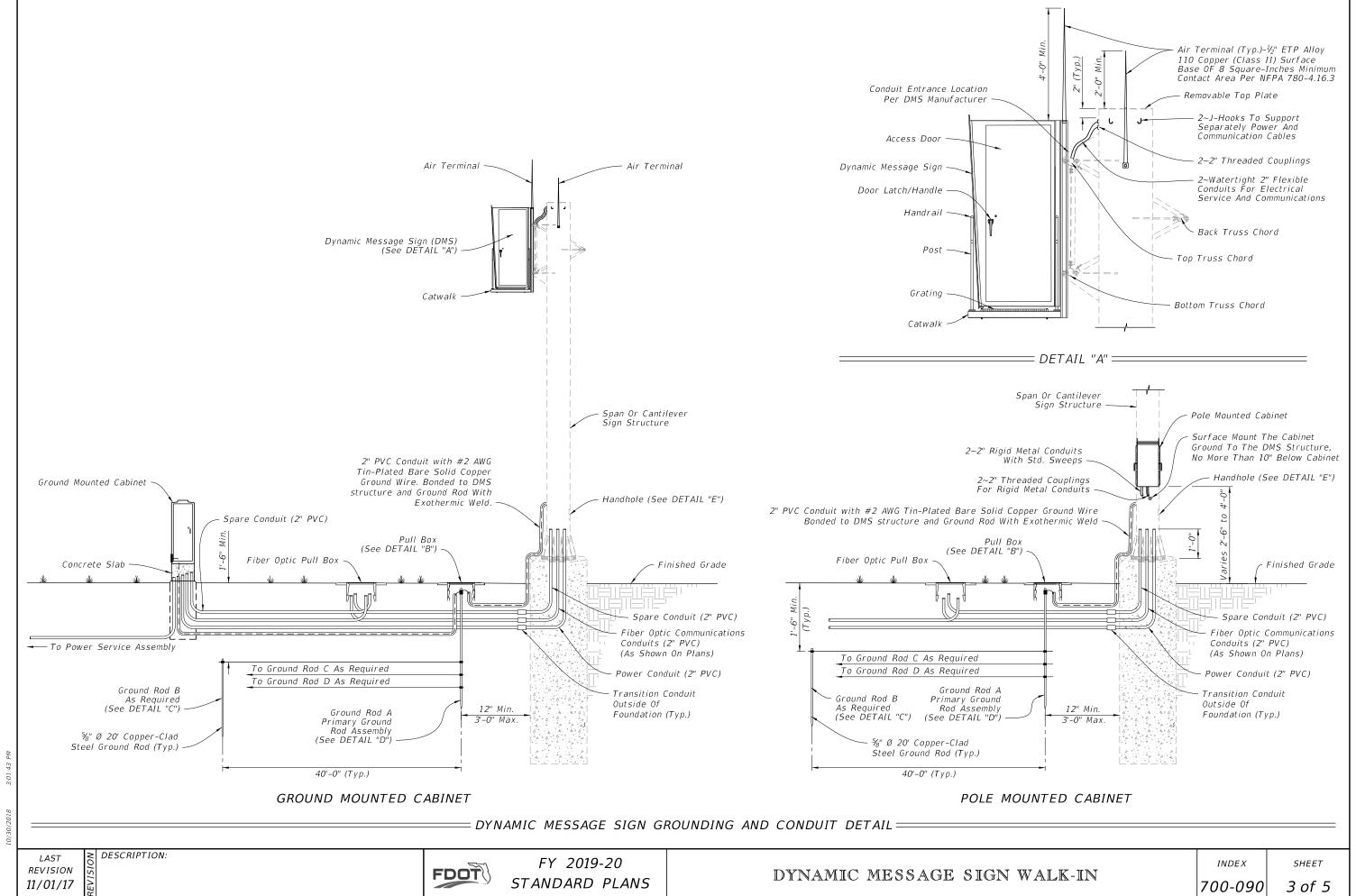
DYNAMIC MESSAGE SIGN WALK-IN

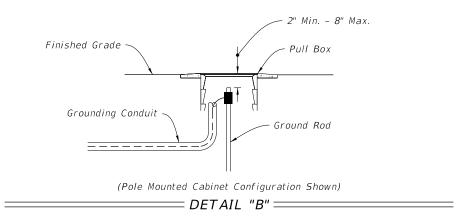
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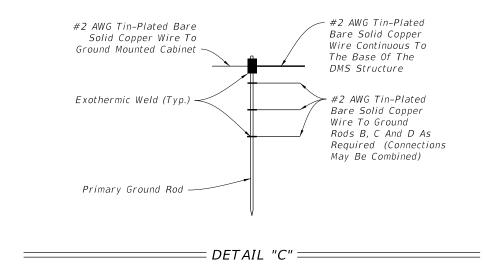
SHEET

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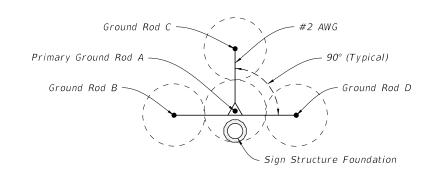








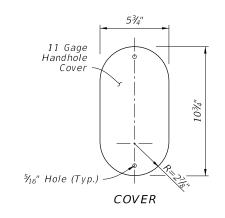
20' Radius Each "Sphere Of Influence"

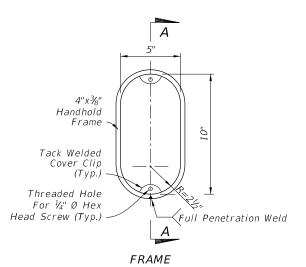


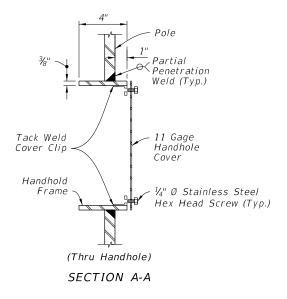
TYPICAL (20' Rods, 40' Spacing)

GROUND ROD ARRAY DETAIL

= DETAIL "D" =







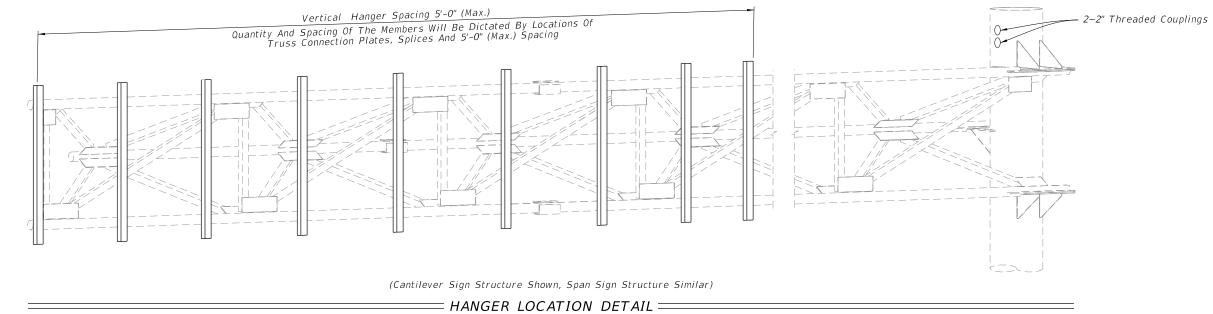
DETAIL "E"

LAST REVISION 11/01/17

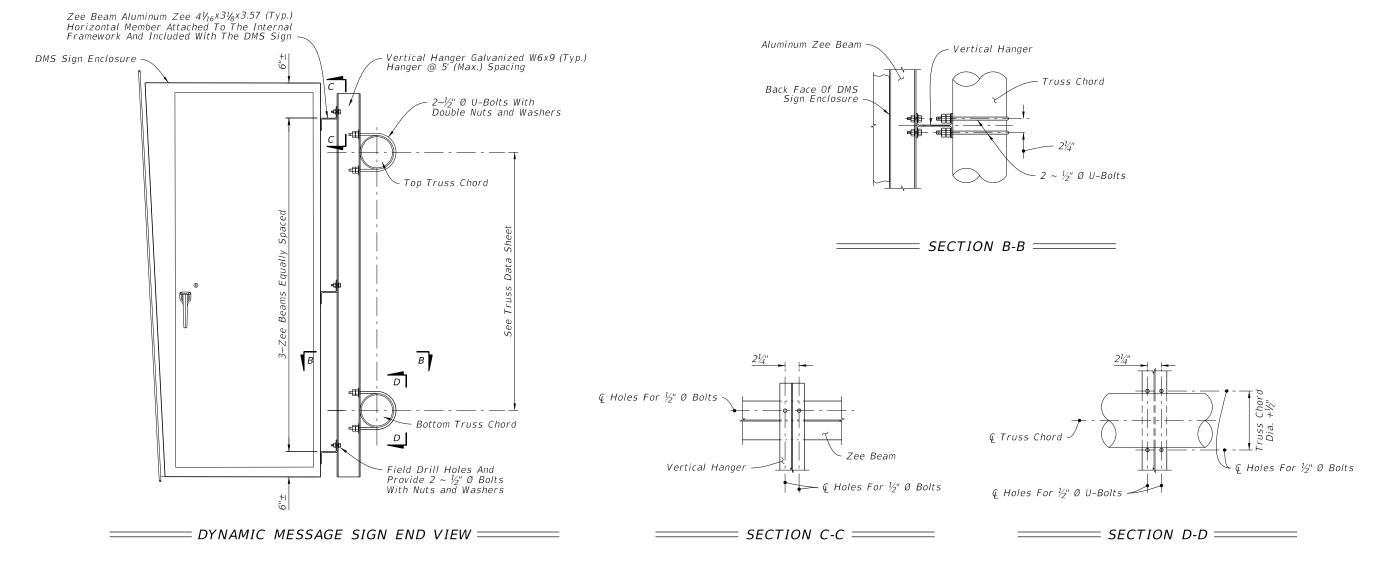
DESCRIPTION:

FDOT

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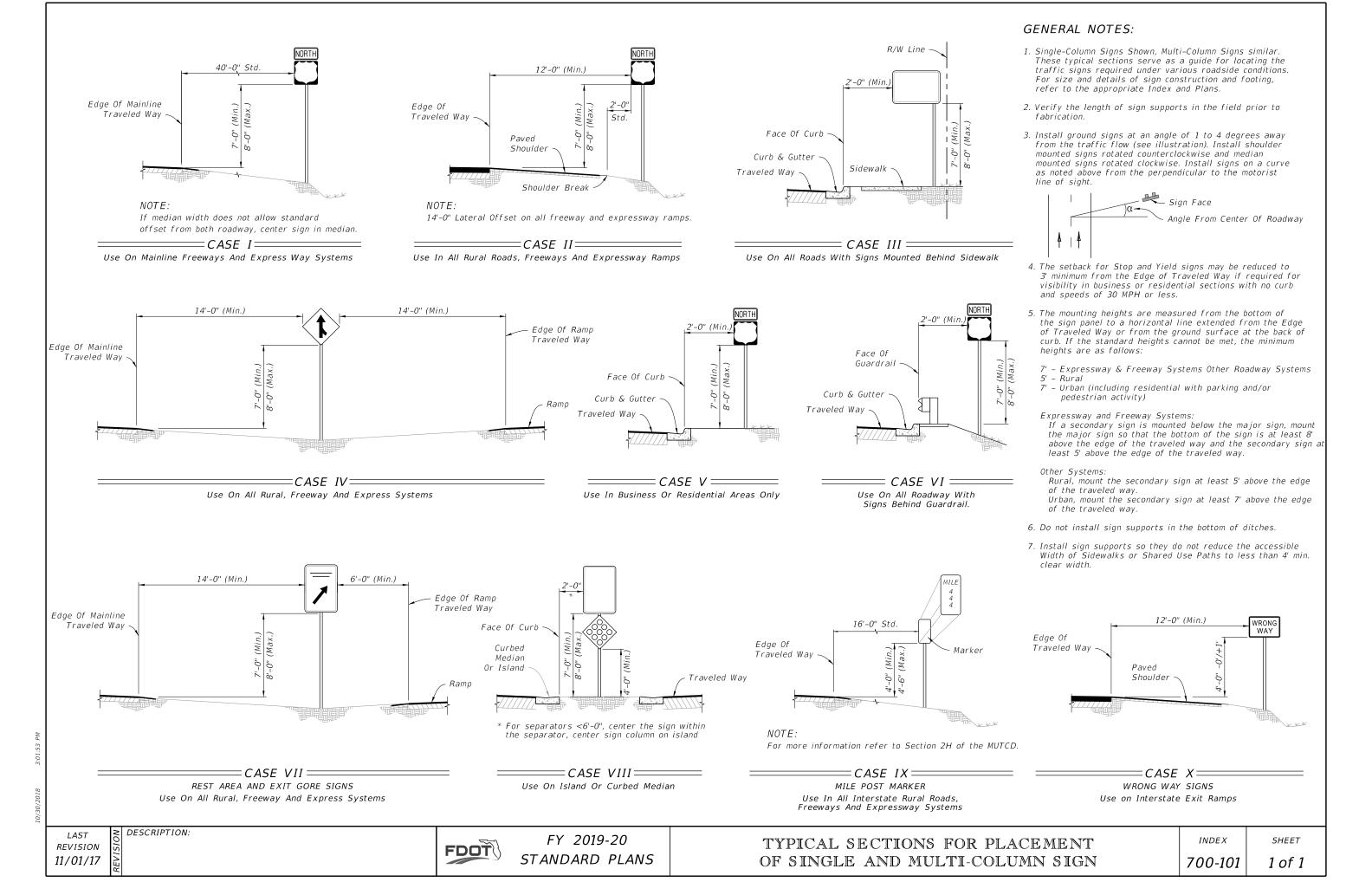
_____ HANGER LOCATION DETAIL

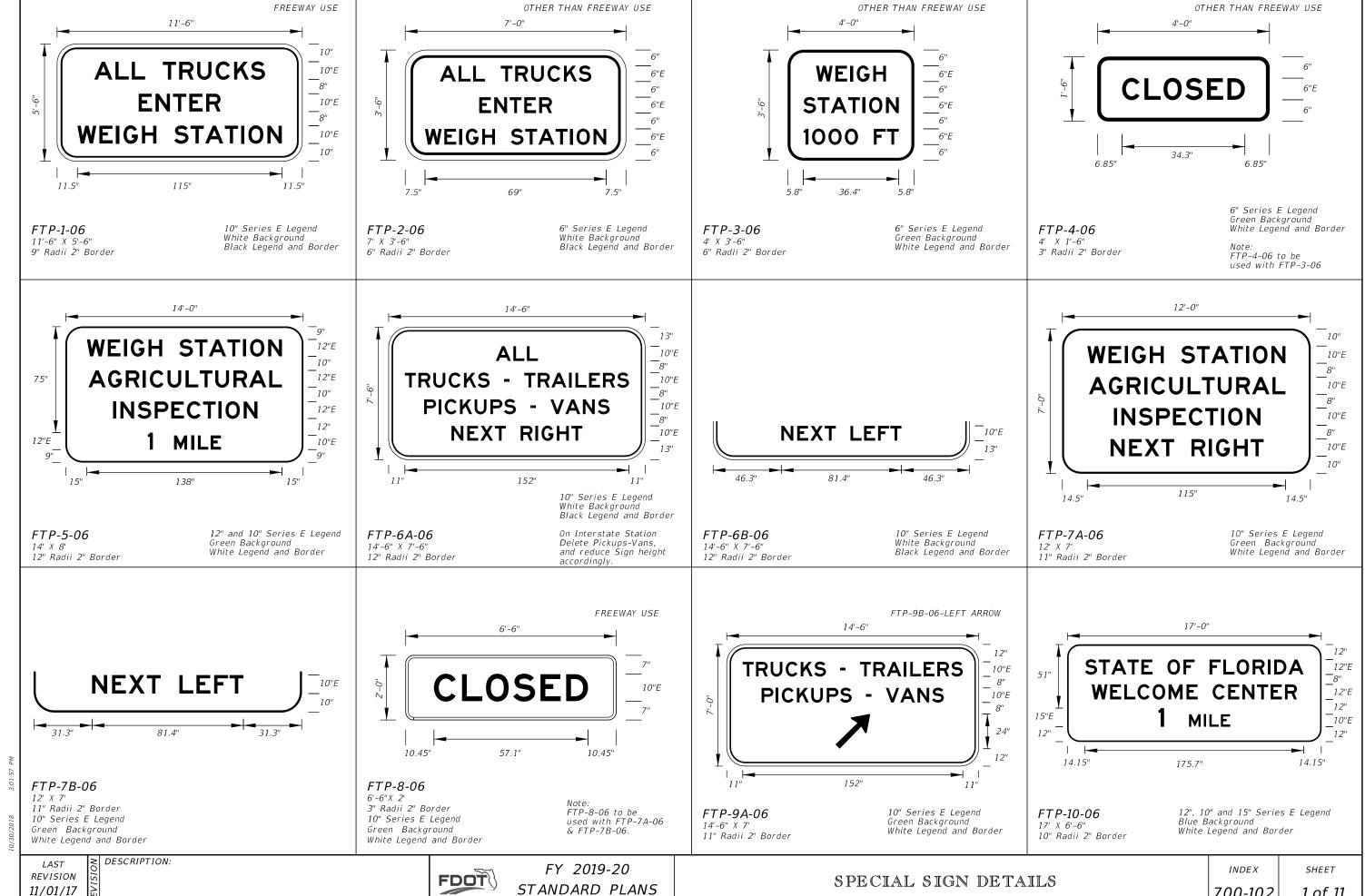


10/30/2018

LAST OUS DESCRIPTION:

FDOT





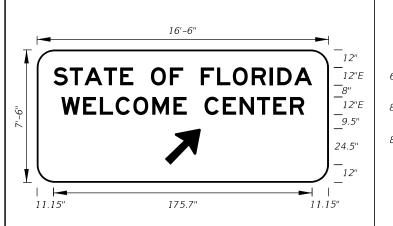
FDOT

STANDARD PLANS

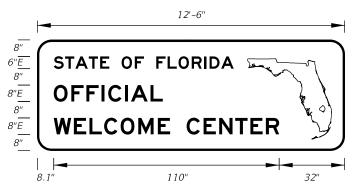
SPECIAL SIGN DETAILS

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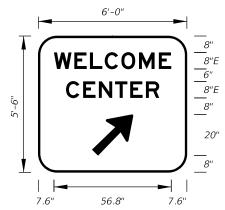
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FTP-11-06 16'-6" X 7'-6" 12" Radii 2" Border 12" Series E Legend Blue Background White Legend and Border



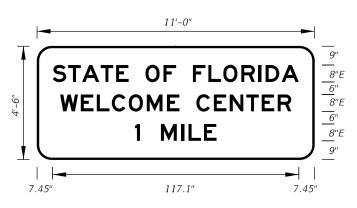
FTP-12-06 12'-6" X 4'-6" 7" Radii 2" Border 6" and 8" Series E Legend Blue Background White Legend and Border



FTP-13-06 6' 0" X 5'-6" 9" Radii 2" Border 8" Series E Legend Blue Background White Legend and Border



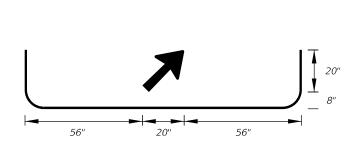
FTP-14-06 16'-0" X 7'-0" 11" Radii 2" Border 13.3 and 10" Series E Legend Blue Background White Legend and Border



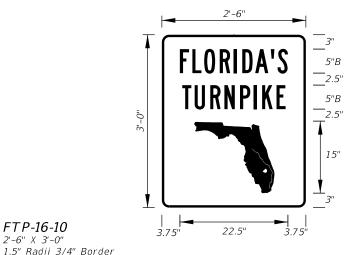
FTP-15A-06 11'-0" X 4'-6" 7" Radii 2" Border 8" Series E Legend Blue Background White Legend and Border



FTP-15B-06 11'-0" X 5'-0" 8" Radii 2" Border 8" and 12" Series E Legend Blue Background White Legend and Border



FTP-15C-06 11'-0" X 5'-6" 9" Radii 2" Border 8" Series E Legend Blue Background White Legend and Border



FTP-17-10 3'-0" X 4'-0" 1.5" Radii 3/4" Border 7" Series B Legend Green Background White Legend, Border, and Florida Symbol



4'-0" 8"B TURNPIKE 8"B 4" 25.6" 37.2"

and Florida Symbol DESCRIPTION: **REVISION** 11/01/17

FTP-16-10

2'-6" X 3'-0"

5" Series B Legend

White Legend, Border,

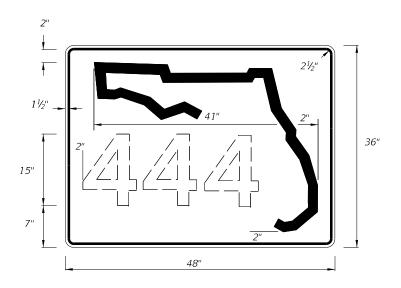
Green Background

FDOT

FY 2019-20 STANDARD PLANS FTP-18-10 4'-0" X 5'-0" 3" Radii 1 1/4" Border 8" Series B Legend Green Background White Legend, Border, and Florida Symbol

INDEX

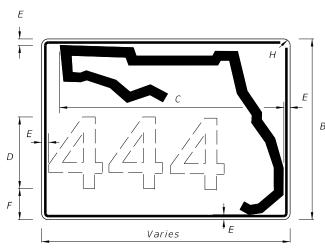
SHEET



DIGITS	NUMERAL SIZE	SERIES LEGEND	PANEL SIZE
1-3	15"	С	48" x 36"
4	12"	С	48" x 36"

- 1. Stroke width of State Outline shall be 1".
- 2. 2½" Radii

# INDEPENDENT USE FOR FREEWAY = 1 OR 2 DIGITS



3 OR MORE DIGITS

### NOTES:

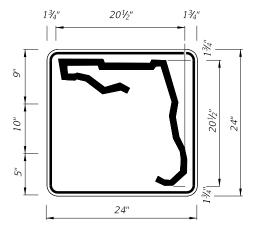
- Florida marker shall have Black Legend with White Background.
- 2. Stroke width of State outline shall be 1¾" for Guide Sign.
- 3. Series D Legend.
- 4. ¾" Border

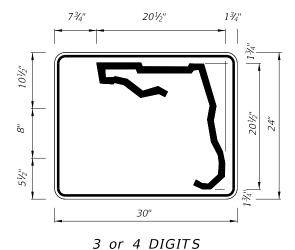
DESCRIPTION:

### D Н 30" 24" 26" 11/4" 12" 23/4" 81/4" 11/4" 31/4" 11/4" 83/4" 38" 11"

### GUIDE SIGN USE

=FTP-17-06 - FLORIDA ROUTE MARKER=





1 or 2 DIGITS

DIGITS	NUMERAL SIZE	SERIES LEGEND	PANEL SIZE	DIGITS	-
1-2	10"	D	24" x 24"	3	

DIGITS	NUMERAL SIZE	SERIES LEGEND	PANEL SIZE	
3	8"	D	30" x 24"	

30" x 24"

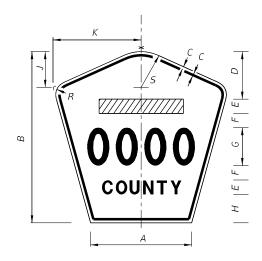
### NOTES:

- 1. Stroke width of State Outline shall be 1".
- 2. The 24" X 24" panel shall only be used for a 3 digit route when the panel is to be used on a sign cluster with other 24" X 24"
- 3. 1½" Radii

### INDEPENDENT USE OTHER THAN FREEWAY =

### NOTES:

- 1. Series D Legend.
- 2. Color: Yellow Legend and Border on Blue Background.
- 3. When used on a guide sign, marker must be overlaid on a rectangular Yellow Background as shown in chart.
- 4. When two or more County Route Markers are mounted together, use the dimensions of the largest marker for all other markers.



	DIMENSIONS							Rectangular					
SIGN	А	В	С	D	Ε	F	G	Н	J	К	R	5	Yellow Background
4 DIGIT POST MOUNTED	251/8"	42"	3/4"	10"	4"	4"	8"	8"	8¾"	22"	5"	8¾"	Dimensions (See Note 3)
2 DIGIT OVERHEAD	21½"	36"	1/2"	71/2"	3"	3"	12"	41/2"	71/8"	187/8"	41/4"	7½"	42"x 42"
3 DIGIT OVERHEAD	251/8"	42"	3/4"	8"	4"	4"	12"	6"	83/8"	22"	5"	8¾"	48"x 48"
4 DIGIT OVERHEAD	29 ⁷ /8"	48"	3/4"	8"	5"	5"	12"	8"	93/4"	25%"	5¾"	101/4"	52"x 52"

= FTP-18-06 - COUNTY ROUTE MARKER (M1-6)=

LAST **REVISION** 11/01/17

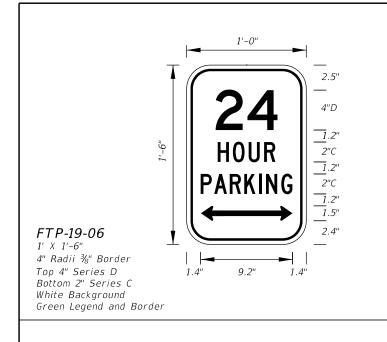
FDOT

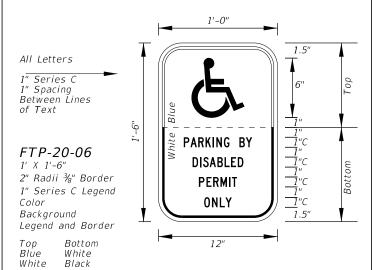
FY 2019-20 STANDARD PLANS

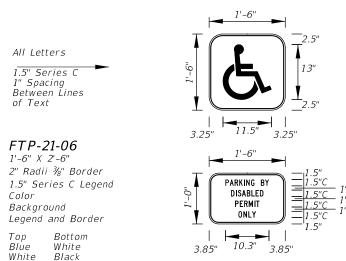
SPECIAL SIGN DETAILS

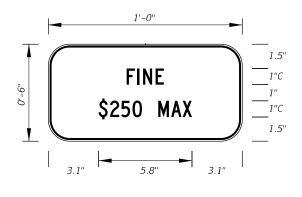
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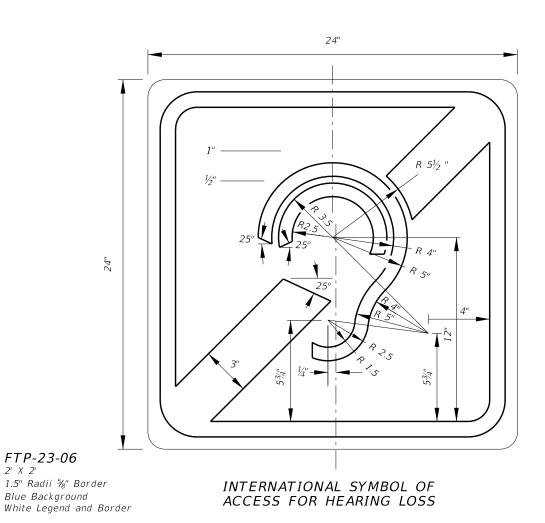


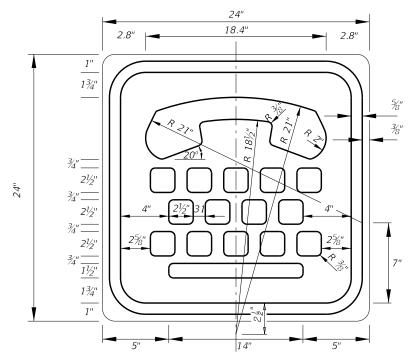


FTP-22-06

1' X 6" 1" Radii ¾" Border

1" Series C Legend White Background Black Legend and Border Supplemental Panel for the FTP-20-06 and FTP-21-06 signs





FTP-24-06 2' X 2' 1.5" Radii ¾" Border Blue Background White Legend and Border

INTERNATIONAL TDD SYMBOL

REVISION 11/01/17

FTP-23-06 2' X 2'

DESCRIPTION:

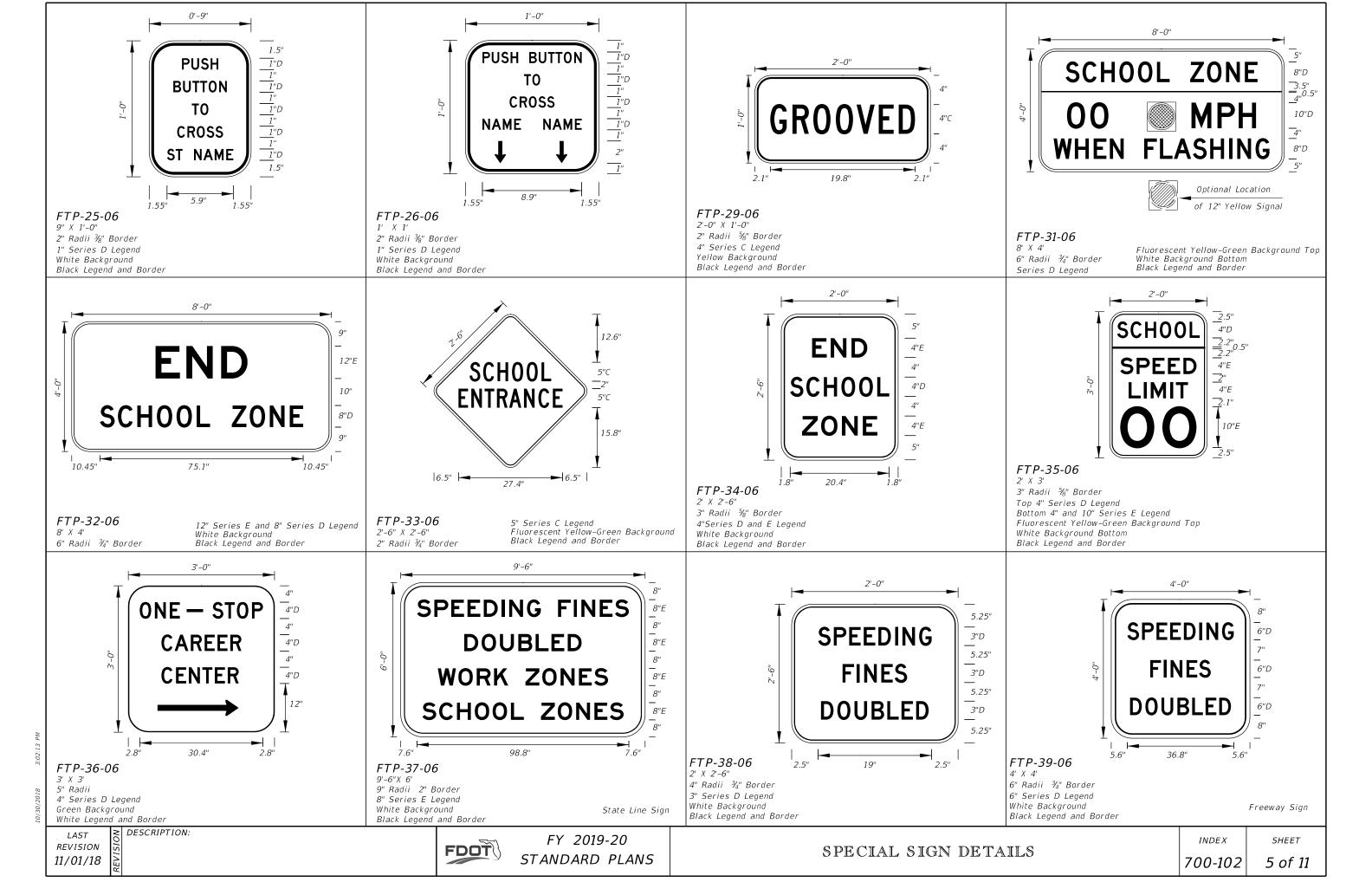
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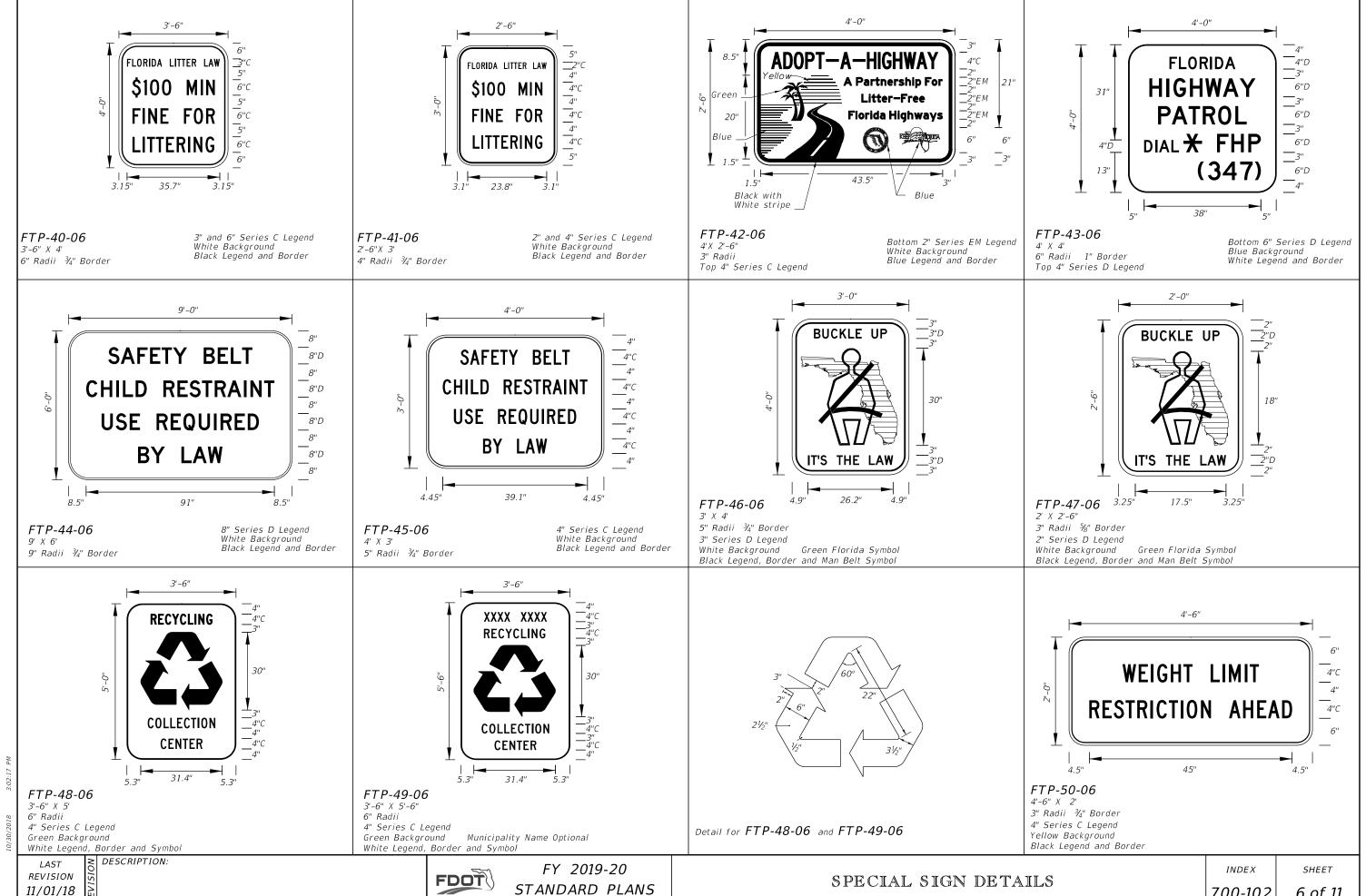
FY 2019-20 STANDARD PLANS

SPECIAL SIGN DETAILS

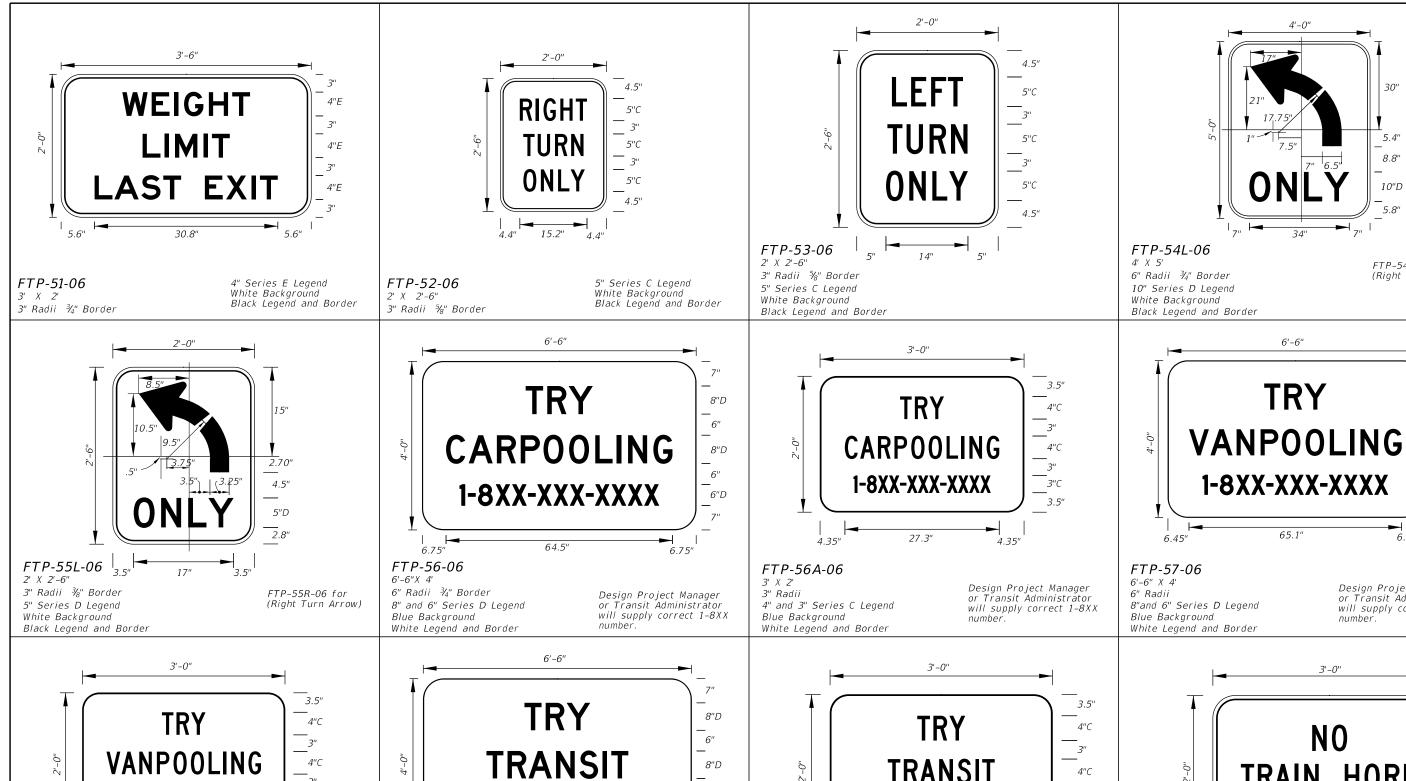
INDEX 700-102

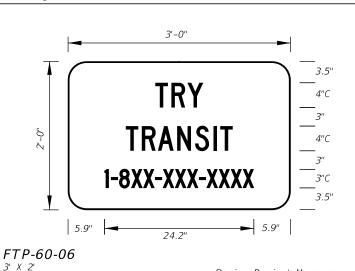
SHEET





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# N₀ 4"C TRAIN HORN 4"C 3"C 4.5' 4.3"

**TRY** 

65.1"

3' X 2'

4" and 3" Series C Legend

FTP-61-06

Black Legend and Border

3" Radii ¾" Border Yellow Background

White Legend and Border **REVISION** 11/01/18

FTP-58-06

Blue Background

3' X 2'

3" Radii

FDOT

FTP-59-06

Blue Background

8" and 6" Series D Legend

White Legend and Border

6'-6" X 4'

6" Radii

FY 2019-20 STANDARD PLANS

number.

1-8XX-XXX-XXXX

8"D

6"D

3" Radii

4"and 3" Series C Legend

White Legend and Border

Blue Background

10.65"

Design Project Manager

or Transit Administrator

will supply correct 1-8XX

SPECIAL SIGN DETAILS

Design Project Manager

or Transit Administrator

will supply correct 1-8XX

INDEX 700-102

SHEET 7 of 11

DESCRIPTION:

4" and 3" Series C Legend

4.25"

1-8XX-XXX-XXXX

27.5"

_____3"C

3.5"

4.25"

Design Project Manager

or Transit Administrator

will supply correct 1-8XX

10"D

FTP-54R-06 for

(Right Turn Arrow)

8"D

6"

6"

Design Project Manager or Transit Administrator

will supply correct 1-8XX

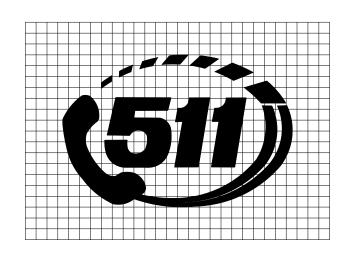
6"D

8"D

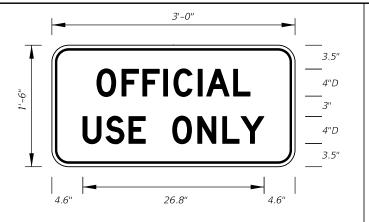


### FTP-62-06 3' X 3'

2" Radii ¾" Border 4"and 5" Series C Legend Yellow Background Black Legend and Border



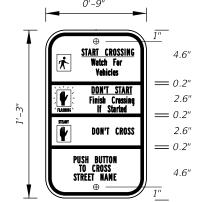
DETAIL for FTP-66 AND FTP-67



### FTP-65-06

3' X 1'-6" 2" Radii ¾" Border 4" Series D Legend White Background Black Legend and Border

Sign Mounting Holes Can Be Punched Or Field Drilled With No Obstruction To Text Or Symbols From Holes Or Bolts.



### FTP-68A-06

FTP-71-06

Black Legend and Border

4' X 4'

1.5" Radii ¾" Border Series B Legend White Background Black Legend and Border

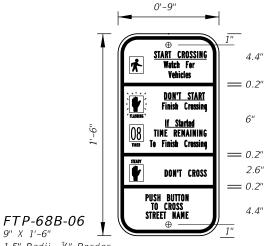
See Standard Highway Signs Manual, Sign R10-3b For Letter Size Spacing And Symbol Sizes.



FTP-66-06 2" Radii ¾" Border

7" Series D Legend Blue Background White Legend and Border

Sign Mounting Holes Can Be Punched Or Field Drilled With No Obstruction To Text Or Symbols From Holes Or Bolts.



9" X 1'-6" 1.5" Radii ¾" Border Series B Legend White Background Black Legend and Border

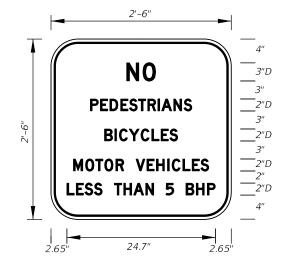
See Standard Highway Signs Manual, Sign R10-3b For Letter Size Spacing And Symbol Sizes.



FTP-67-06 3' X 4'

2" Radii ¾" Border 5" Series D Legend

Blue Background White Legend and Border



FTP-69-06 2'-6" X 2'-6" 4" Radii ¾" Border 2" and 3" Series D Legend White Background Black Legend and Border



FTP-70-06 3'-6" X 2'-6"

2.25" Radii ¾" Border

5" Series C and 7" Series C Legend Blue Background White Legend and Border

DESCRIPTION:

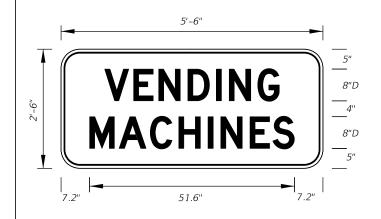
**FIRE SMOKE** AREA 29" 18.3" 18.3" 2" Radii ¾" Border 8" Series C Legend Yellow Background

14.8' 8"C 8"C 6" 8"C 14.8"

FTP-72-06 3' X 3' 2" Radii ¾" Border

12.3 **FIRE SMOKE** 6"C 6"C 12.3"

> 6" Series C Legend Yellow Background Black Legend and Border



FTP-73-06 5'-6" X 2'-6"

Blue Background White Legend and Border

4" Radii ¾" Border

8" Series D Legend

**REVISION** 11/01/17

FDOT

FY 2019-20 STANDARD PLANS

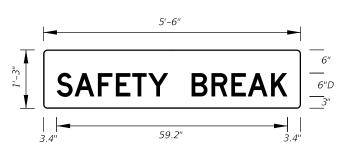
SPECIAL SIGN DETAILS

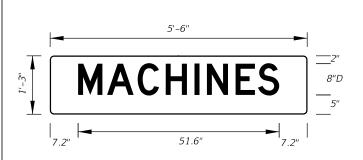
INDEX 700-102

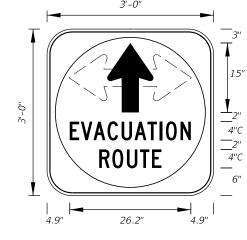
SHEET

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FTP-74-06

5'-6" X 2'-6" 4" Radii ¾" Border 6" Series D Legend Blue Background White Legend and Border FTP-75-06

5'-6" X 1'-3" 1" Radii 6" Series D Legend Blue Background White Legend

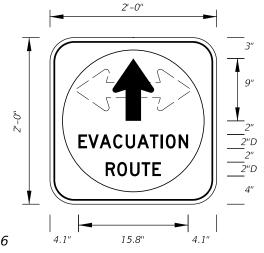
FTP-76-06 5'-6" X 1'-3" 1" Radii 8" Series D Legend Blue Background White Legend

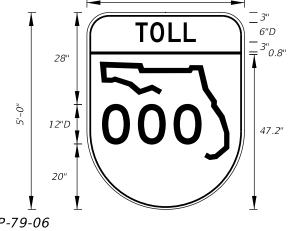
FTP-77-06 3' X 3'

5" Radii ¾" Border 4" Series C Legend

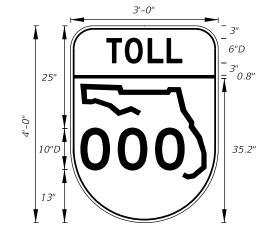
White Background with Blue Circle Background

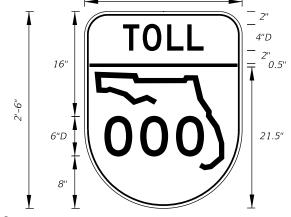
White Legend and Black Border





4'-0"





2'-0"

FTP-78-06 2' X 2'

3" Radii ¾" Border 2" Series D Legend

White Background with Blue Circle Background White Legend and Black Border

FTP-79-06 4' X 5'

6" Radii ¾" Border 6" and 12" Series D Legend

Top Yellow Background with Black Legend and Black Border Bottom White Background with Black Legend and Border

FTP-80-06 3' X 4'

5" Radii ¾" Border

6"and 10" Series D Legend

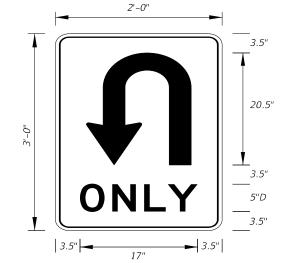
Top Yellow Background with Black Legend and Black Border Bottom White Background with Black Legend and Border

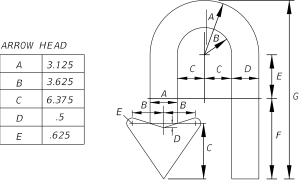


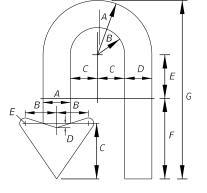
3" Radii ¾" Border

4" and 6" Series D Legend

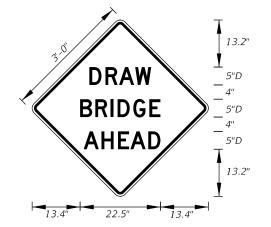
Top Yellow Background with Black Legend and Black Border Bottom White Background with Black Legend and Border







ŀ	10'-0"	1
5'-0"	ALL TRUCKS	9.8" 10"E 8"
<u> </u>		22.4" — 9.8"
1	1.95" 96.1" 11.95	<u> </u> 



FTP-82-08

1.5" Radii 5" Series D Legend White Background Black Legend and Border

DESCRIPTION:

ARROW BODY В D 6.25 3.125 3.125 3.125

FTP-83-08 10'-0" X 5'-0" 8" Radii

10" Series E Legend Green Background White Legend

FTP-84-09 3' X 3' 1.5" Radii

5" Series D Legend Yellow Background Black Legend and Border

**REVISION** 11/01/17

**FDOT** 

FY 2019-20 STANDARD PLANS

9.25

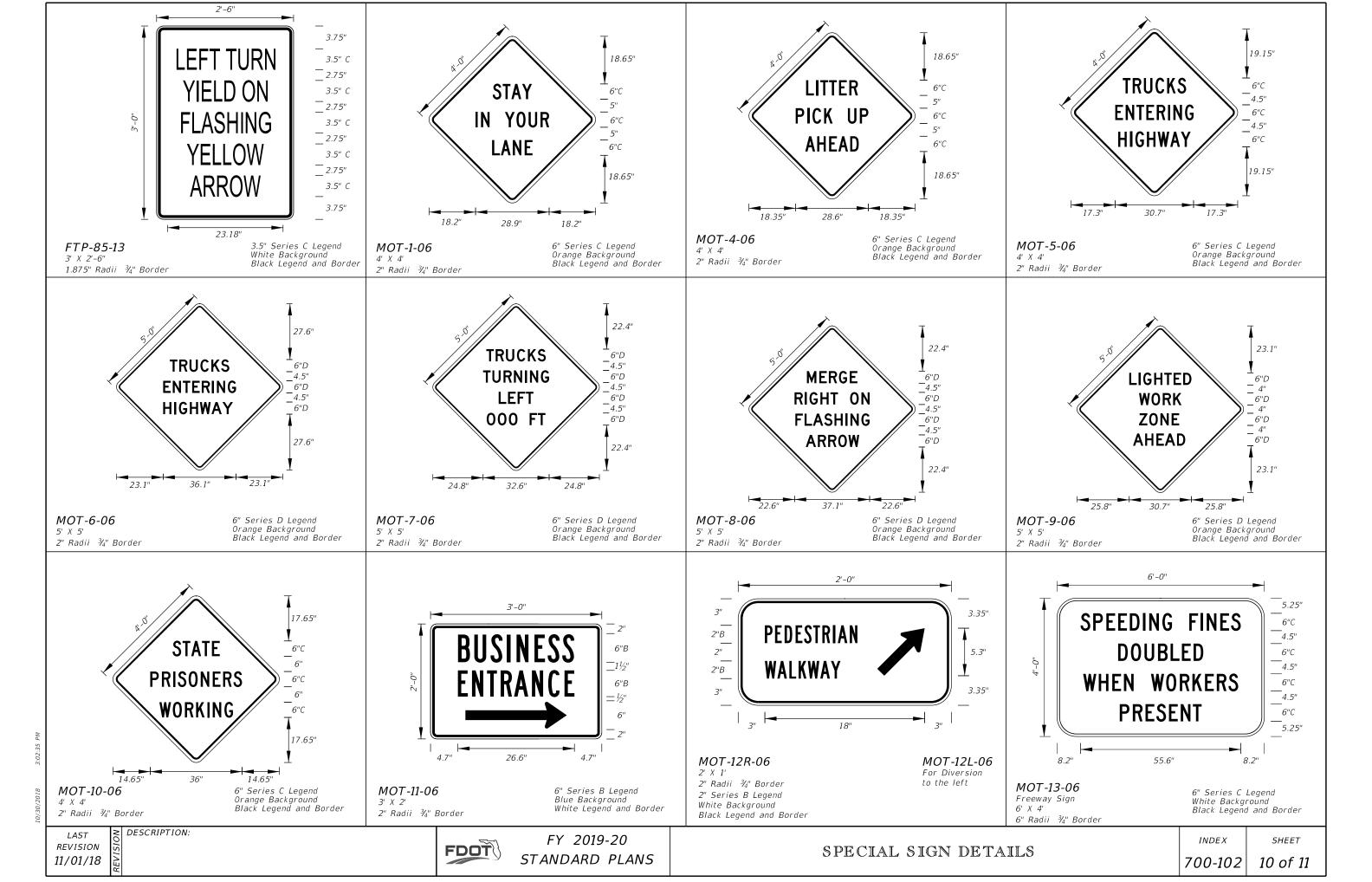
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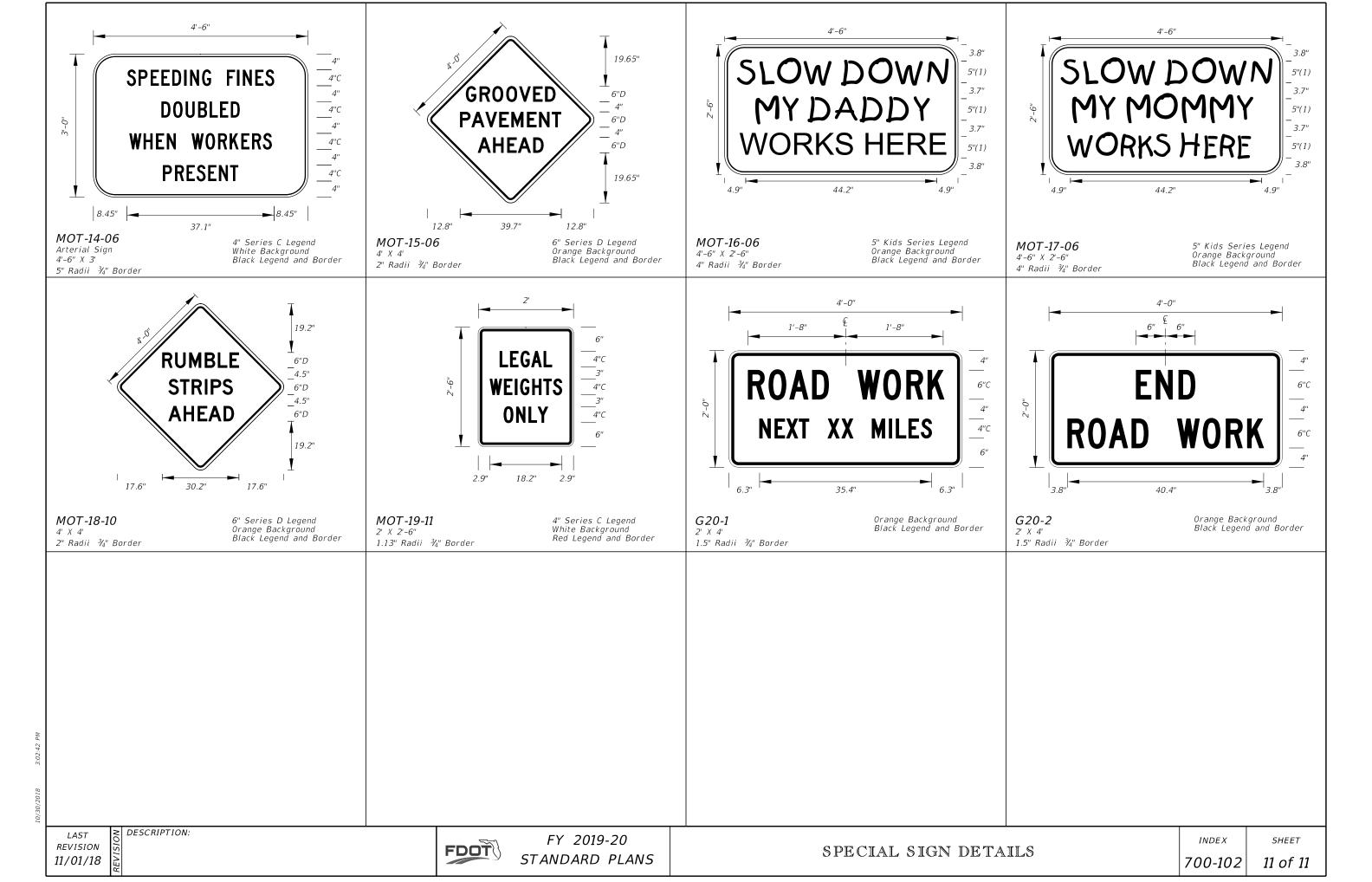
20.5

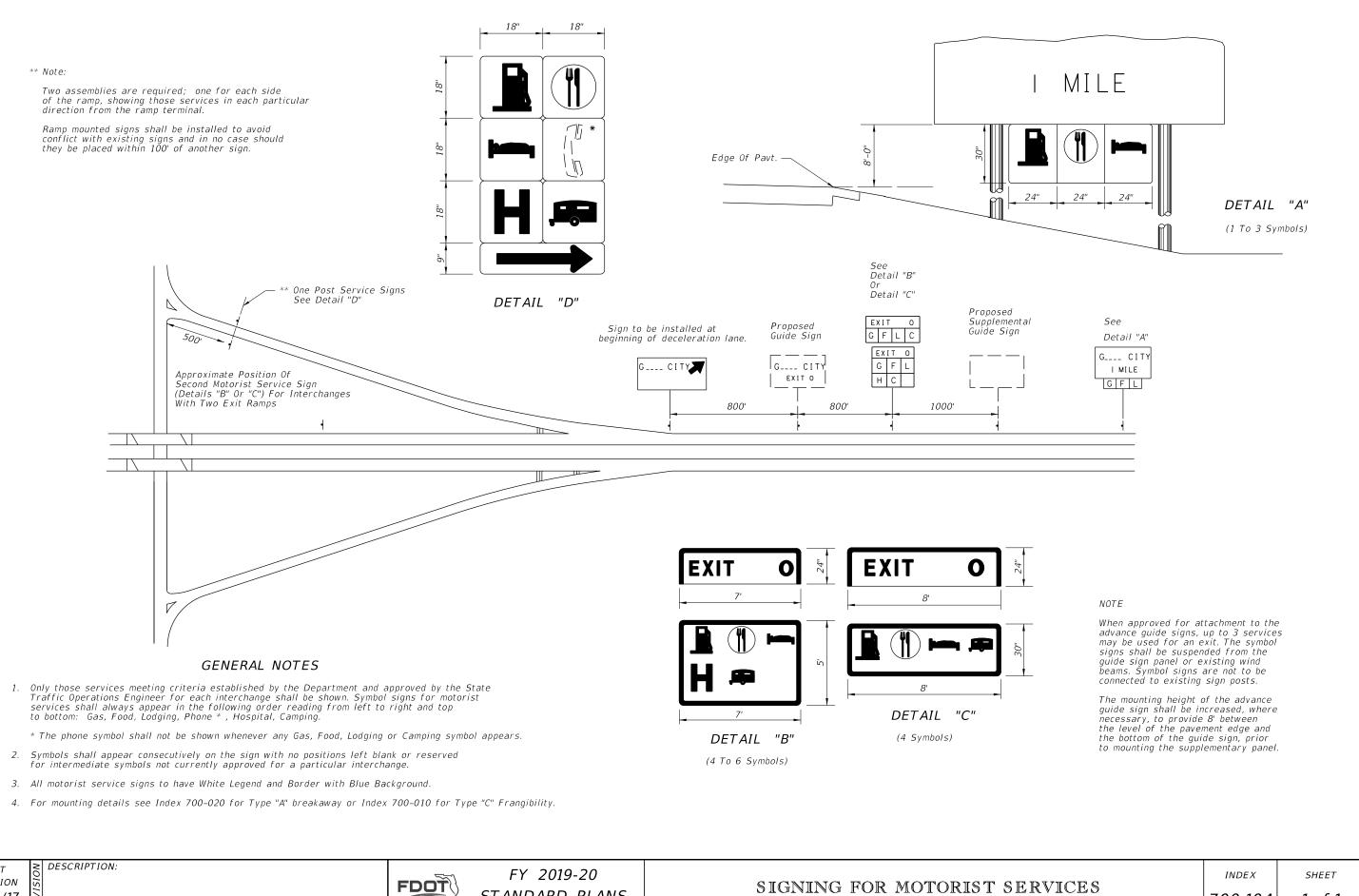
SPECIAL SIGN DETAILS

INDEX SHEET

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**REVISION** 11/01/17

# STATE OF FLORIDA **WELCOME CENTER** MILE

STATE OF FLORIDA **WELCOME CENTER** 

STATE OF FLORIDA **OFFICIAL WELCOME CENTER** 

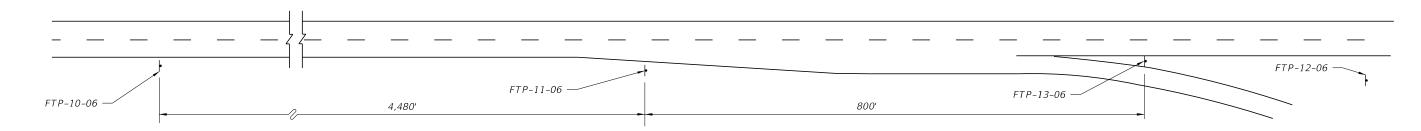


Sign FTP-10-06

Sign FTP-11-06

Sign FTP-12-06

Sign FTP-13-06



Note: Roadway not drawn to scale Distances shown are adequate for driver communication but may be altered slightly if conditions require.

# Tourist Information Center **NEXT RIGHT**

Sign FTP-14-06

Note: Sign FTP-14-06 shall be used as a supplemental guide sign at interchanges which have a Tourist Information Center approved for such signing (locate half-way between normal guide signs)

### Notes:

- 1. Signs and sign structures shall be erected in accordance with the details shown on Index 700-020.
- 2. Sign FTP-12-06 shall be located on the Welcome Center grounds in proximity to the building and as far from the main line roadway as possible (2 signs back to back).
- 3. Sign FTP-10-06, 11-06, 12-06 shall be located as limited access highways only.
- 4. All legend to be Series E.
- 5. See Index 700-102 for sign details.

FOR LIMITED ACCESS HIGHWAYS

**REVISION** 11/01/17

DESCRIPTION:

**FDOT** 

FY 2019-20 STANDARD PLANS

# STATE OF FLORIDA **WELCOME CENTER** 1 MILE

STATE OF FLORIDA 🖘 **OFFICIAL WELCOME CENTER** 

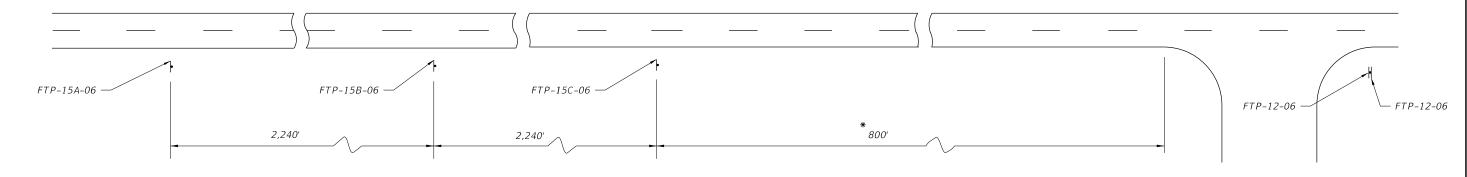
1/2 MILE

SIGN FTP-15B-06

SIGN FTP-15C-06

SIGN FTP-15A-06

SIGN FTP-12-06



* 800' Maximum For Rural Conditions 50' Minimum For Rural Conditions

### Notes:

- 1. Signs and sign structures shall be erected in accordance with the details shown on Index 700-020.
- Sign FTP-12-06 shall be located on the Welcome Center grounds in proximity to the building and as far from the Main Line Roadway as possible (2 signs back to back).
- 3. All legend to be Series E.
- 4. One sign FTP-15A-06 or 15B-06 should be used depending on speed, roadside development & geometric conditions.

FOR PRIMARY HIGHWAYS

**REVISION** 11/01/17

DESCRIPTION:

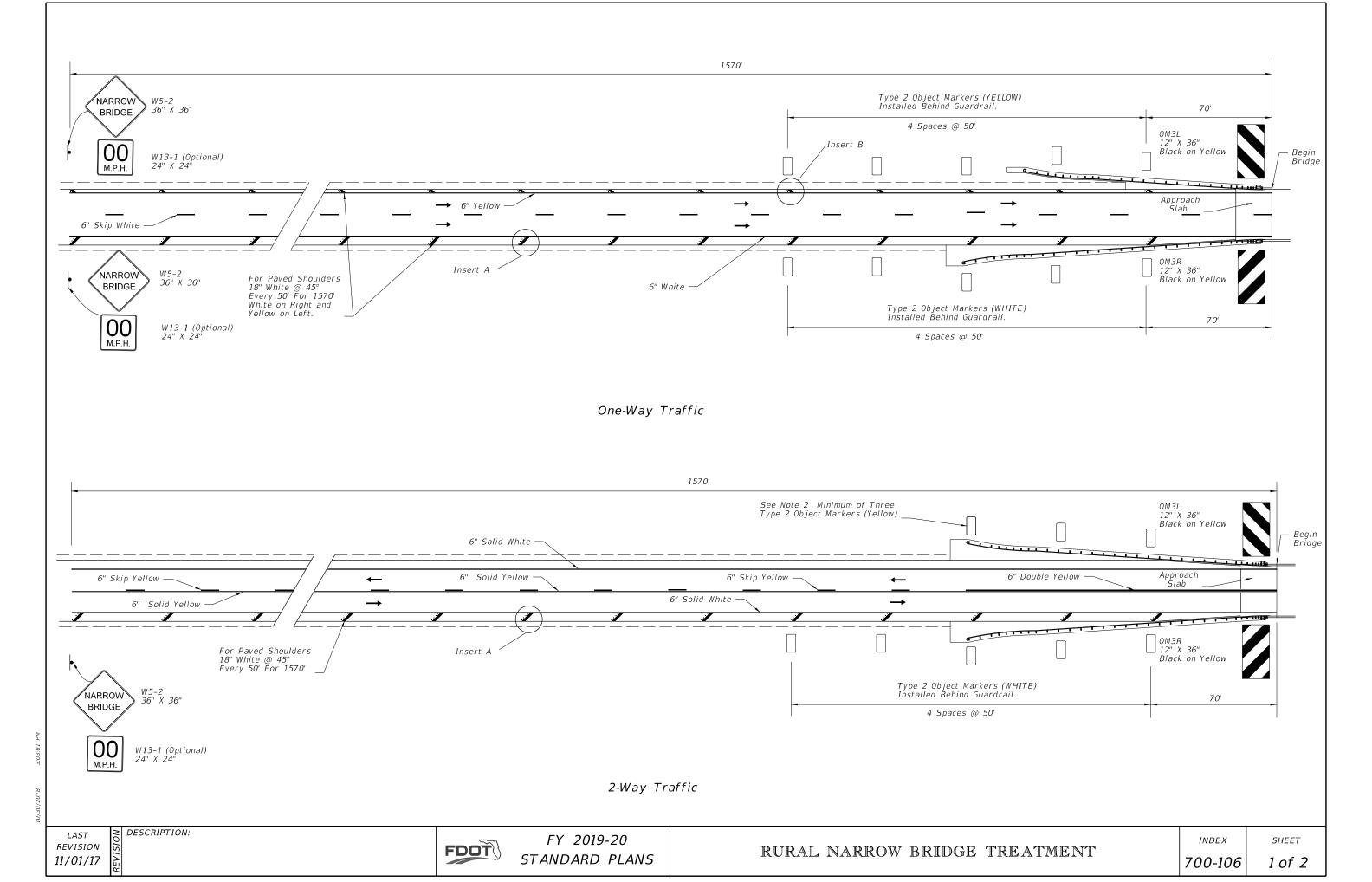
FDOT

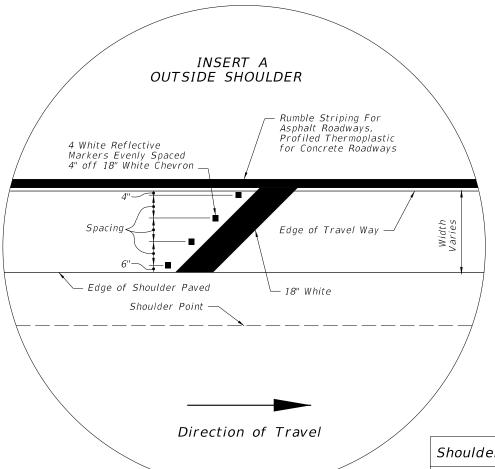
FY 2019-20 STANDARD PLANS

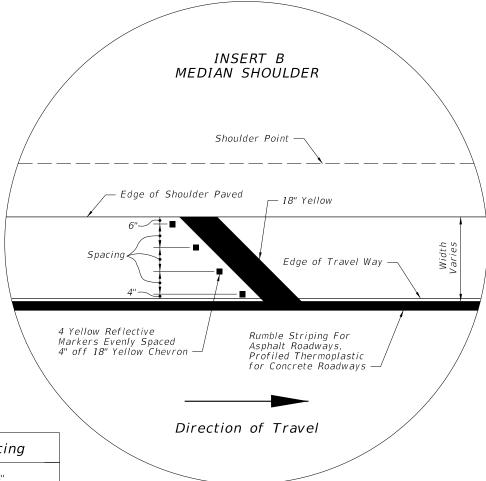
WELCOME CENTER SIGNING

INDEX 700-105

SHEET 2 of 2





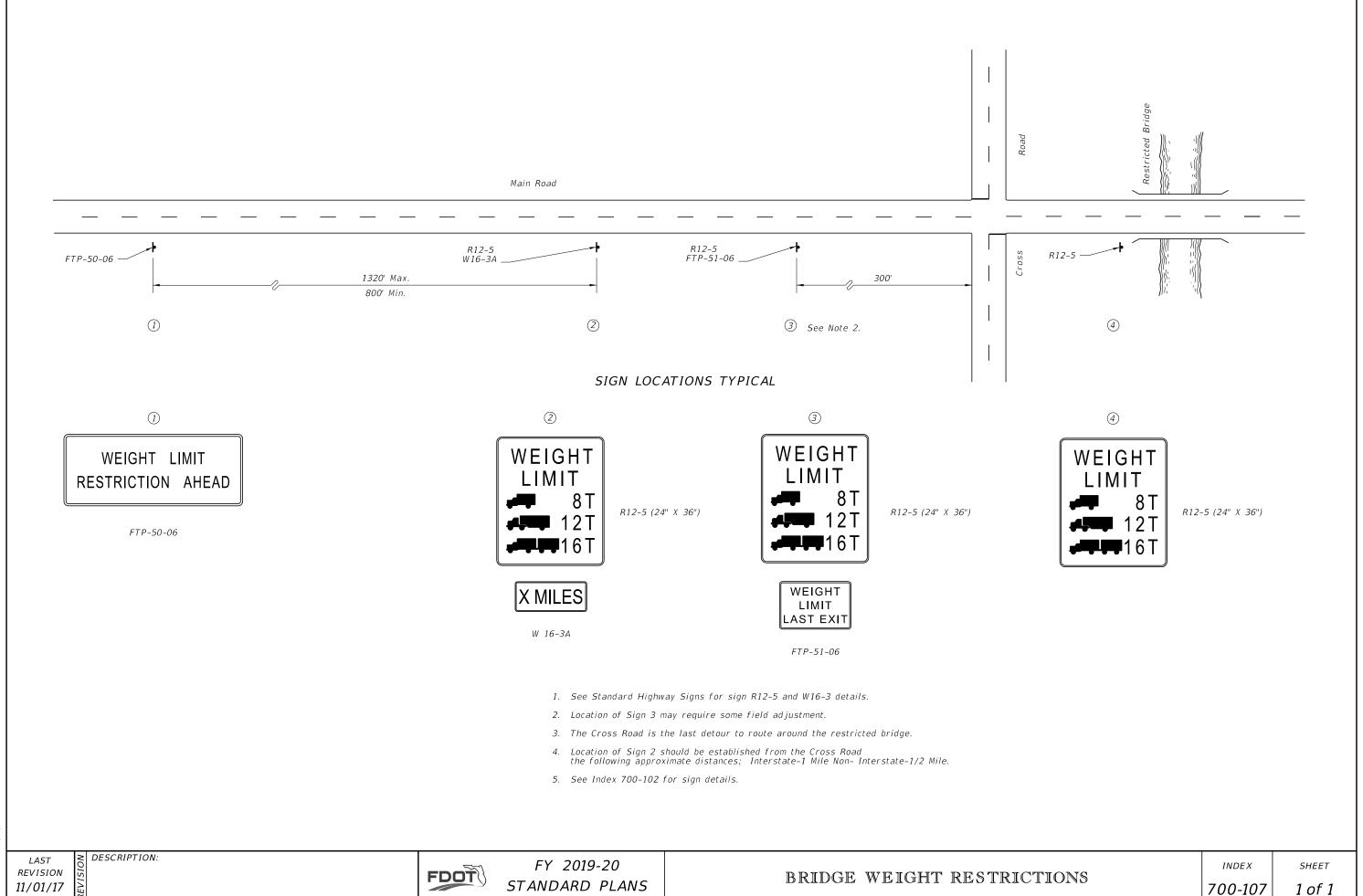


Shoulder Width	No. of RPM's	Spacing
2'	2	14"
3'	3	13"
4'	3	19"
5′	4	16.67"

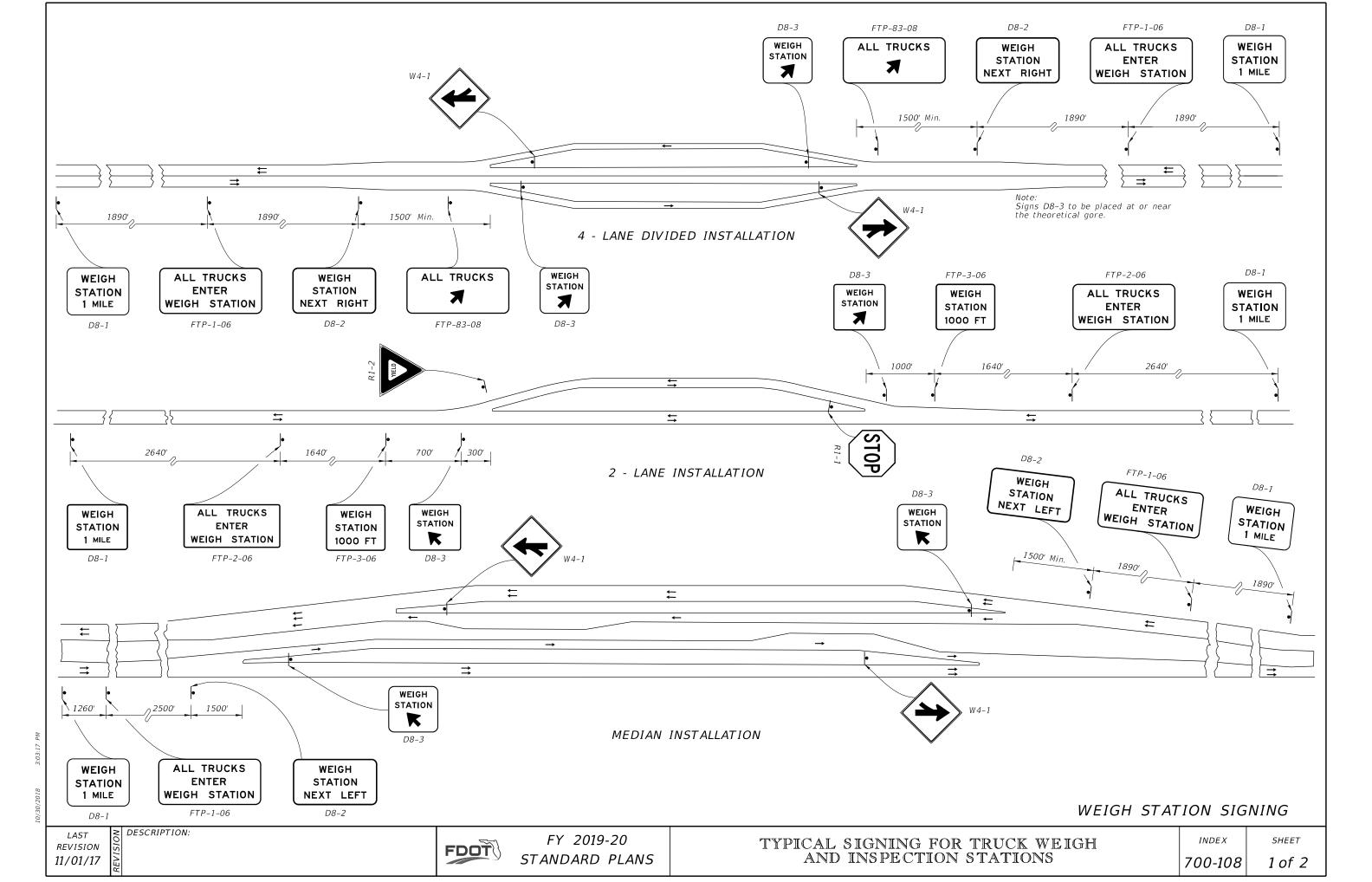
- 1. Roadways with Two-Way Traffic: No passing zone should be extended 1570' in advance of narrow bridge.
- 2. If the bridge or the approach is on a curve, delineators shall be installed for a distance of 1570' in advance of narrow bridge on the outside portion of the roadway. Spacing shall be 100' between delineators. Delineators are to be placed not less than 2' or not more than 8' outside the outer edge of pavement.
- 3. Object markers and delineators on both sides of roadway shall face traffic approaching bridge
- 4. The OM-3R & OM-3L object markers shall be installed 4' above the roadway edge. The panels may be post mounted at the bridges.

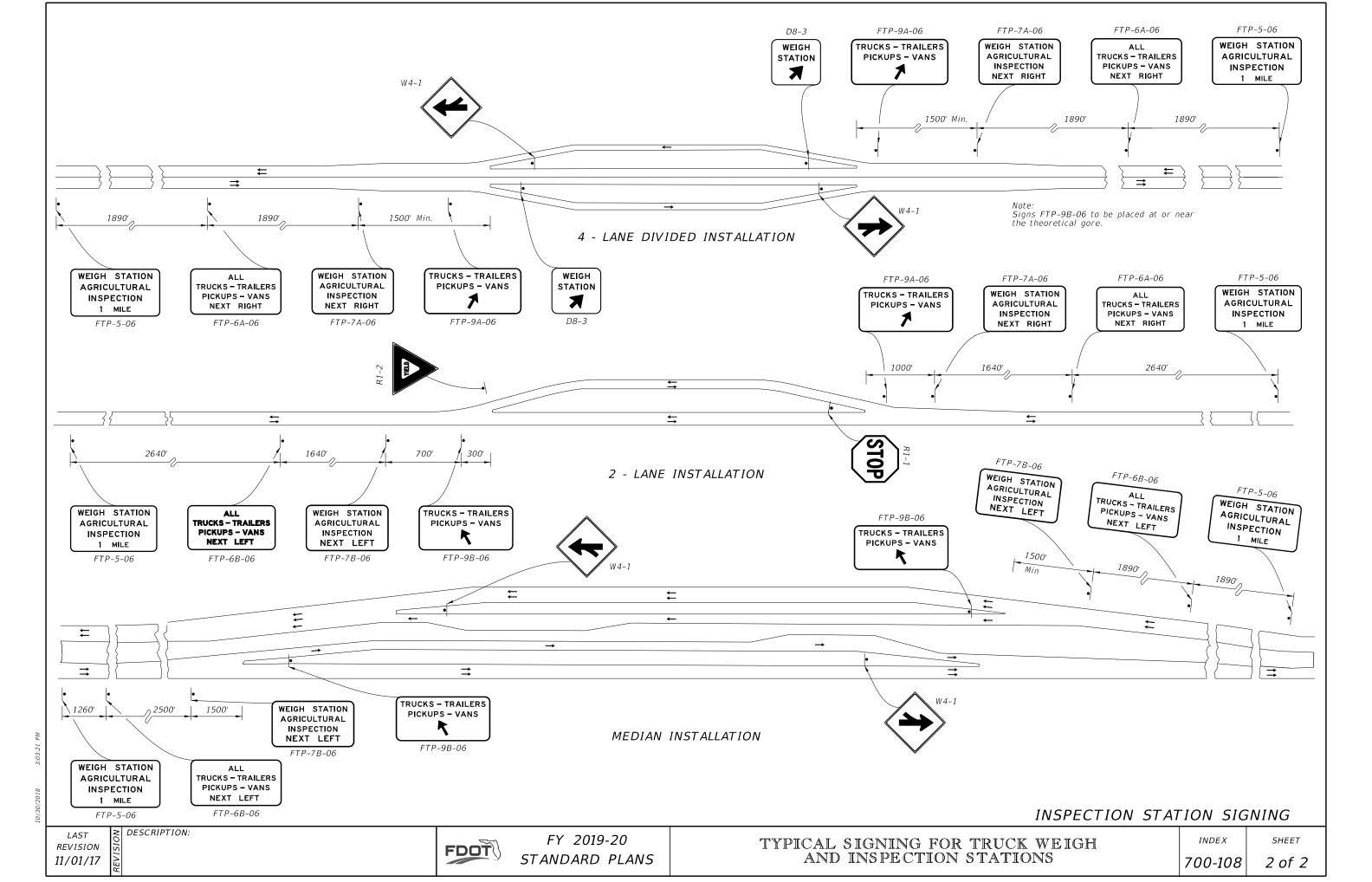
LAST **REVISION** 11/01/17

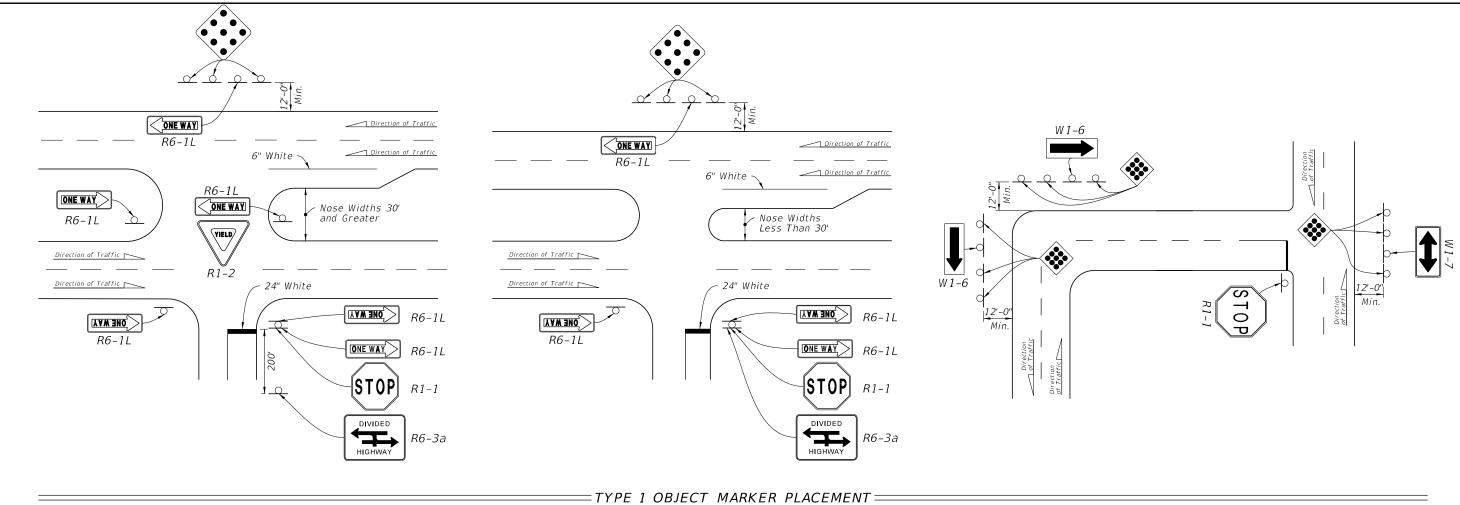
DESCRIPTION:

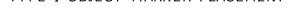


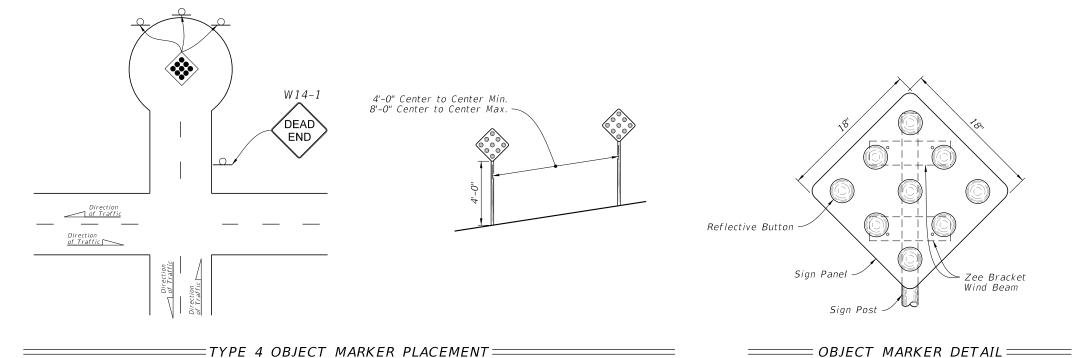
11/01/17











- 1. Index applicable to residential and minor streets only. Major streets to be evaluated on a case-by-case basis.
- 2. Install Object Markers in accordance with Index 700-010
- 3. See Index 711-001 for pavement markings.

REVISION 11/01/18

DESCRIPTION:



FY 2019-20 STANDARD PLANS

TRAFFIC CONTROLS FOR STREET TERMINATIONS

INDEX

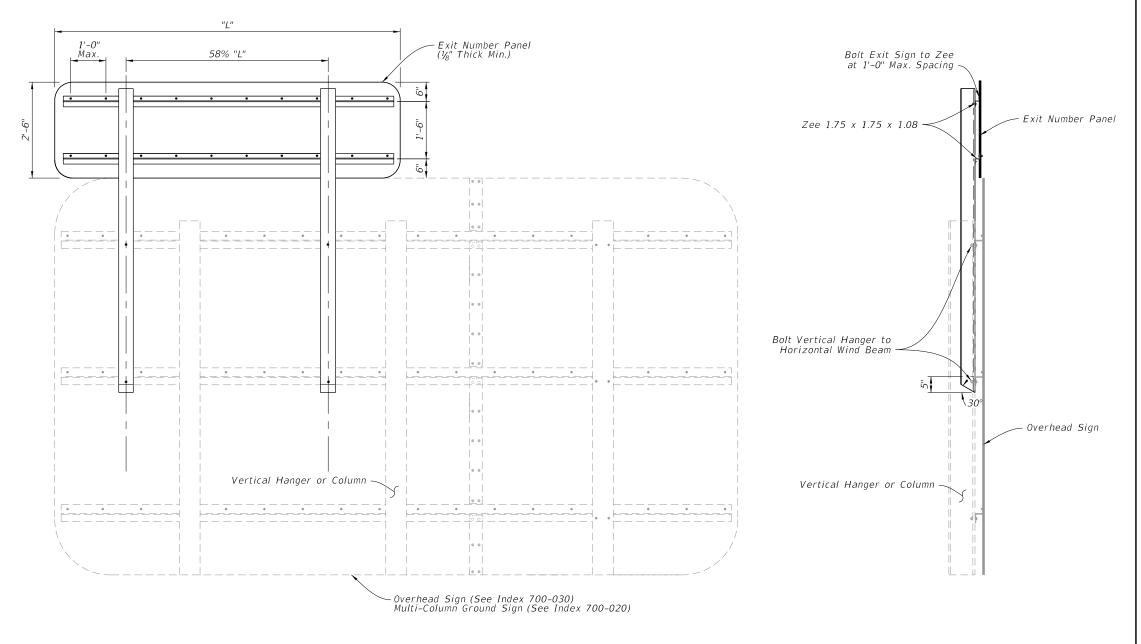
1. Work with Index 700-030.

#### 2. Materials (Aluminum):

- A Sheets and Plates: ASTM B209 Alloy 6061-T6
- B. Standard Structural Shapes: ASTM B308 Alloy 6061-T6
- C. Extruded Shapes: ASTM B221 Alloy 6061-T6
- D. Bolts, Nuts, and Washers:
- a. Bolts: ASTM F468 Alloy 2024-T4 with minimum 0.002-Inch-thick anodic coating, chromate sealed
- b. Washers: ASTM B221 Alloy 2024-T4
- c. Nuts: ASTM F467 Alloy 6061-T6 or 6262-T9

## 3. Fabrication:

- A. See sign layout sheet for dimension "L" and sign face details in the Plans.
- B. Round all sign corners.
- 4. For right exits, install the Exit Numbering Panel to the top right side of the Highway Sign.
- 5. For left exits, install the Exit Numbering Panel to the top left side of the Highway Sign.



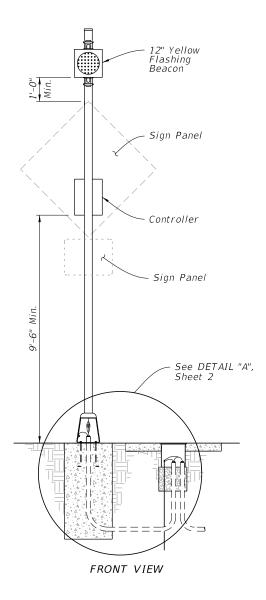
BACK ELEVATION

SIDE ELEVATION

**REVISION** 11/01/18

DESCRIPTION:

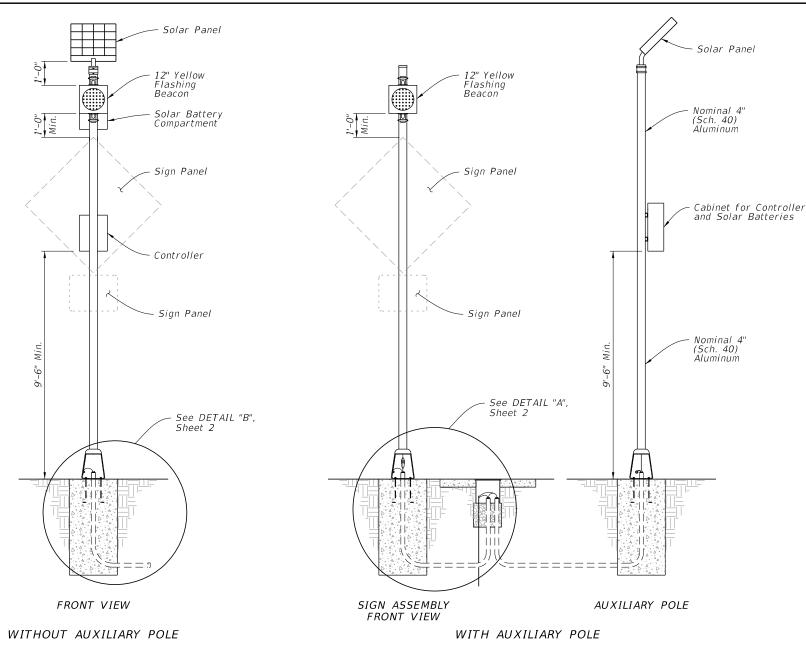






## **GENERAL NOTES:**

- 1. Install sign assemblies based on Alpha-Numeric Type designation shown in the Plans (e.g., Type A1). Assembly Type is based on Power Configuration 'Alpha' Identification shown above and Numerical Identification shown on Sheet 3 thru 8.
- 2. Install sign panel and wind beam in accordance with Index 700-010 and Specification 700.
- 3. Engage all threads on the transformer base and post unless the aluminum post is fully seated into base.
- 4. Meet the requirements of Specification 646 for aluminum poles and transformer bases.
- 5. Install a concrete slab around all roadside assemblies on slopes 6:1 or greater. The minimum slab dimension is 4'-0" by 5'-0".
- 6. When wire entry holes are drilled in the sign column, use a bushing or rubber grommet to protect conductors.

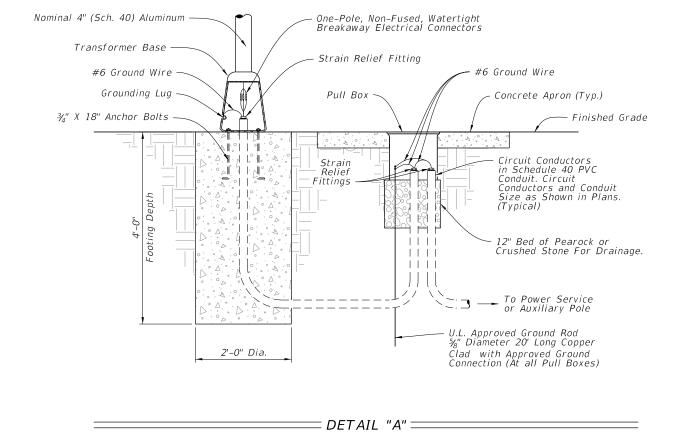


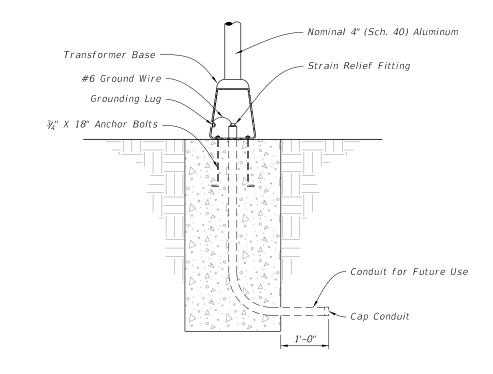
POWER CONFIGURATION 'B' SOLAR-POWERED (Type B1 Shown)

#### POWER CONFIGURATION 'B' NOTES:

- 1. Install a separate pole for mounting the solar panel, controller and batteries for all roadside assemblies with solar panels, controllers and batteries weighing more than 170 lbs.
- 2. Install the auxiliary pole as close to the right of way boundary
- 3. Install the auxiliary pole so that the height is the same as the column for the roadside assembly.
- 4. Orient solar panel to face South for optimal exposure to sunlight.
- 5. The controller and the solar batteries may be located in the same compartment.

TABLE OF CONTENTS:					
Sheet Description					
1	General Notes and Contents				
2	Conduit, Wiring, and Foundation Details				
3	Roadside Sign Assembly-1				
4	Roadside Sign Assembly-2				
5	Roadside Sign Assembly-3				
6	Roadside Sign Assembly-4				
7	Roadside Sign Assembly-5				
8	Roadside Sign Assembly-6				
9	Overhead Sign Assembly				





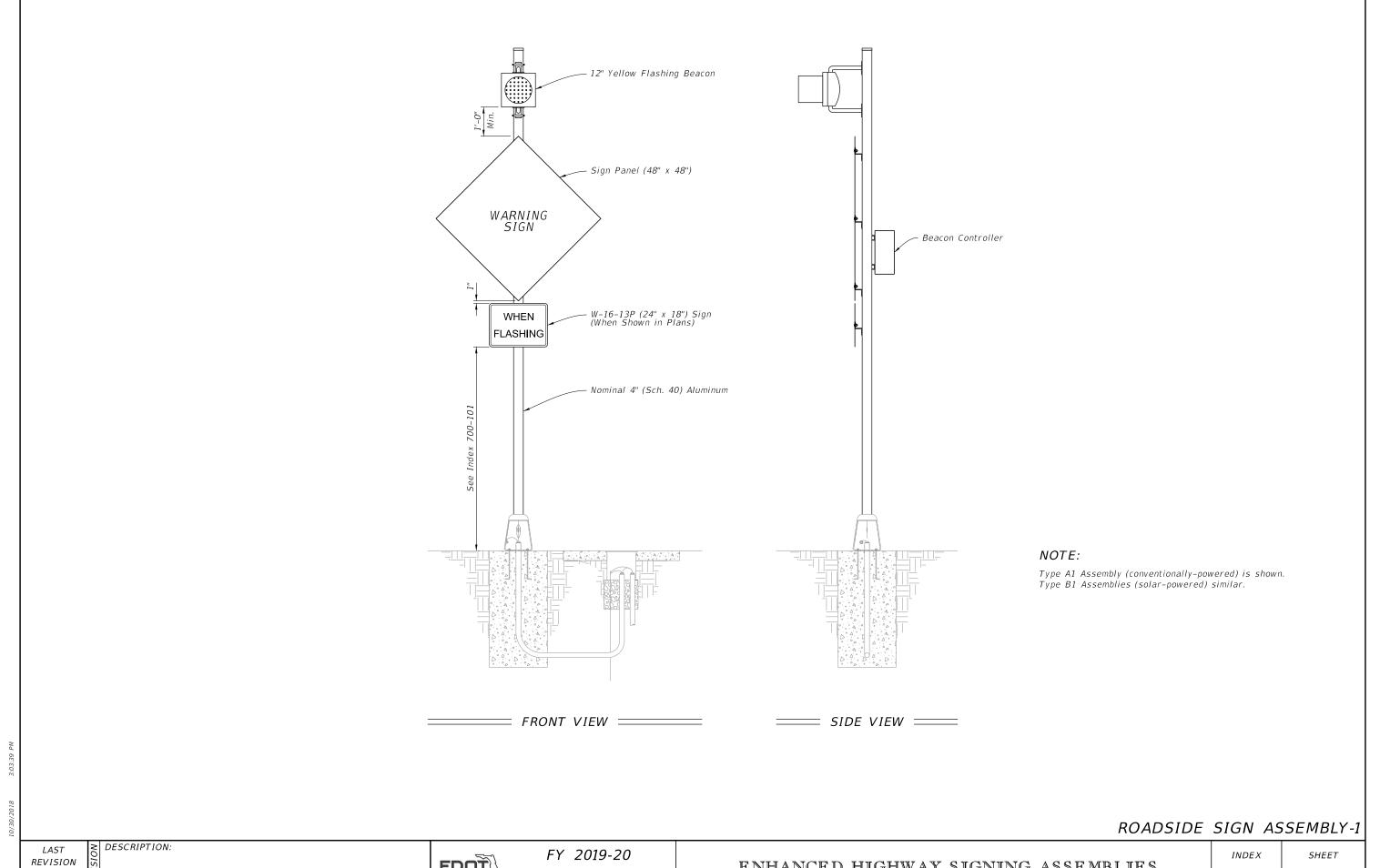
_____ DETAIL "B" ==

CONDUIT, WIRING, AND FOUNDATION DETAILS

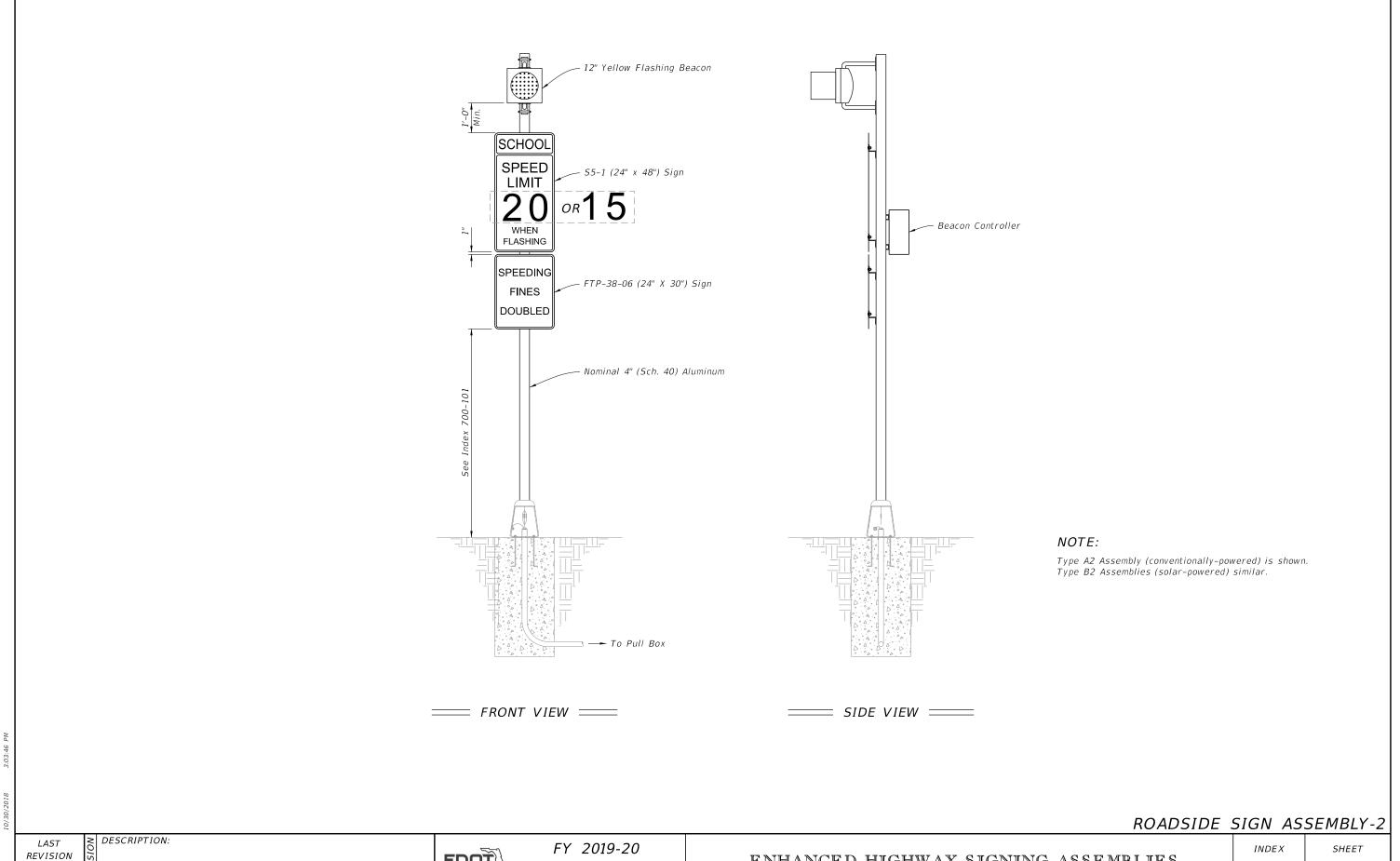
LAST REVISION 11/01/18

DESCRIPTION:

FDOT

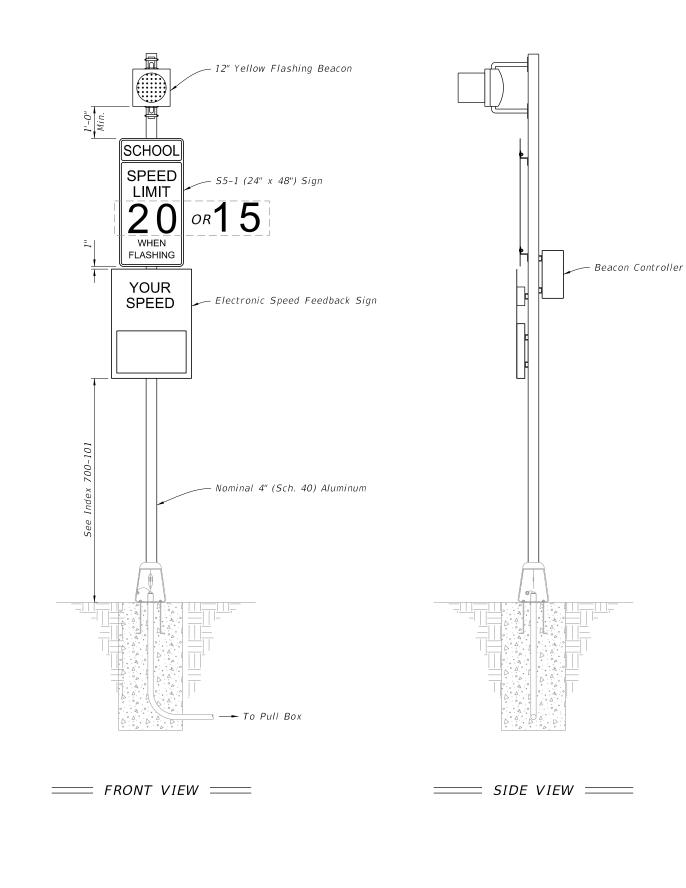


11/01/18



11/01/18

FDOT



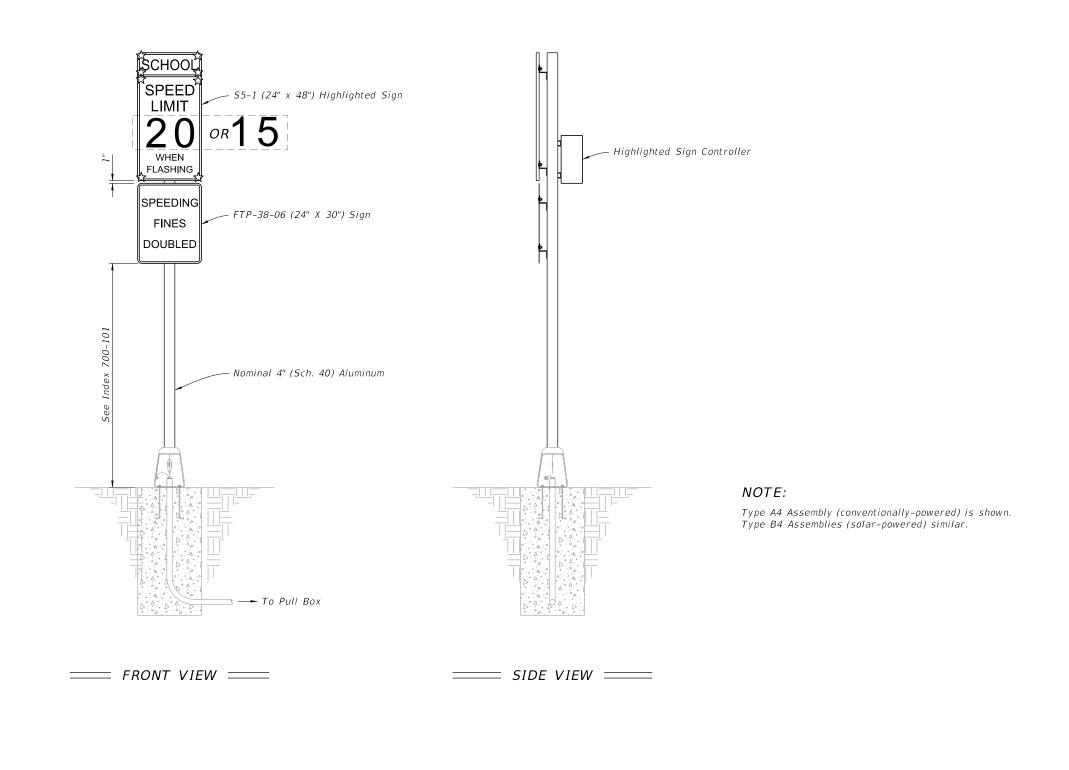
- 1. Type A3 Assembly (conventionally-powered) is shown. Type B3 Assemblies (solar-powered) similar.
- 2. Use electronic speed feedback sign with 15" high numerals for posted speed of 45 mph or less, and 18" high numerals for posted speeds greater than 45 mph.

ROADSIDE SIGN ASSEMBLY-3

DESCRIPTION: REVISION 11/01/18

FDOT

FY 2019-20 STANDARD PLANS

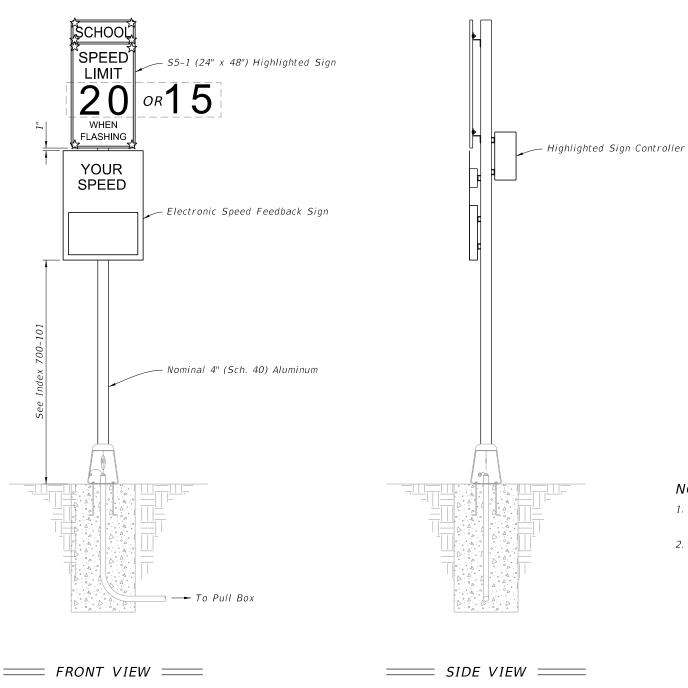


REVISION 11/01/18

≥ DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS



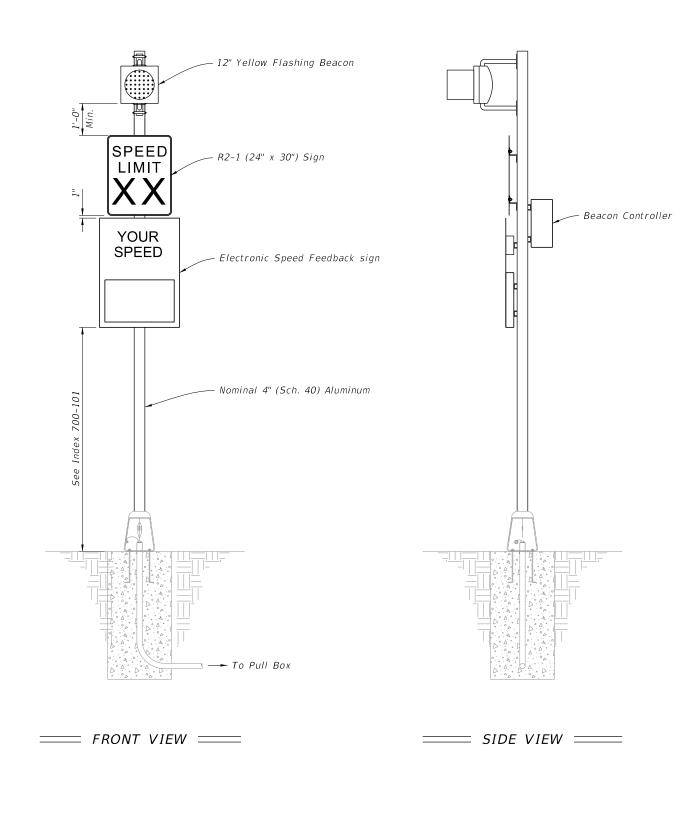
- 1. Type A5 Assembly (conventionally-powered) is shown. Type B5 Assemblies (solar-powered) similar.
- 2. Use electronic speed feedback sign with 15" high numerals for posted speed of 45 mph or less, and 18" high numerals for posted speeds greater than 45 mph.

ROADSIDE SIGN ASSEMBLY-5

DESCRIPTION: REVISION 11/01/18

FDOT

FY 2019-20 STANDARD PLANS

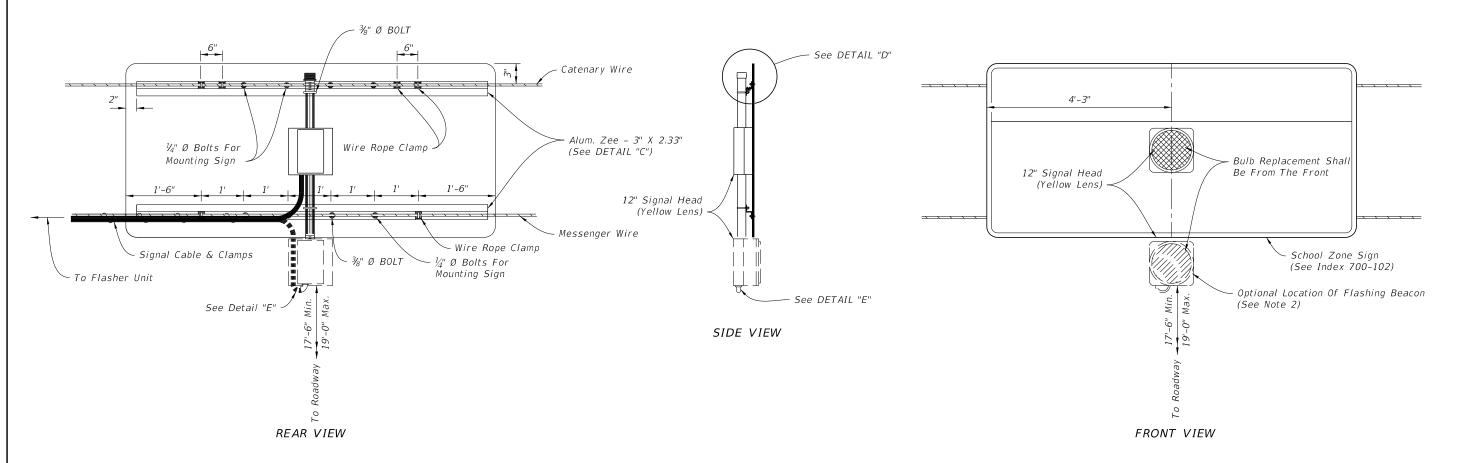


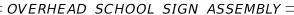
- 1. Type A6 Assembly (conventionally-powered) is shown. Type B6 Assemblies (solar-powered) similar.
- 2. Use electronic speed feedback sign with 15" high numerals for posted speed of 45 mph or less, and 18" high numerals for posted speeds greater than 45 mph.

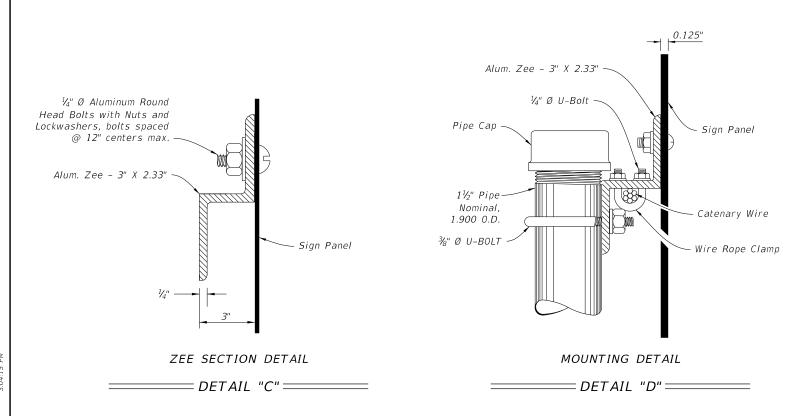
ROADSIDE SIGN ASSEMBLY-6

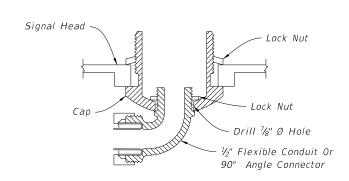
≥ DESCRIPTION: REVISION 11/01/18

FDOT









CABLE ENTRY DETAIL

## **NOTES:**

- 1. Flasher unit and cabinet to be placed on the strain pole supporting overhead sign assembly or on service pole. The flasher unit not to overhang private property or sidewalk.
- 2. Optional flashing beacon will be called for in the Plans. They may be placed within or below the panel, or face to the rear.

# OVERHEAD SIGN ASSEMBLY

**REVISION** 11/01/18

DESCRIPTION:

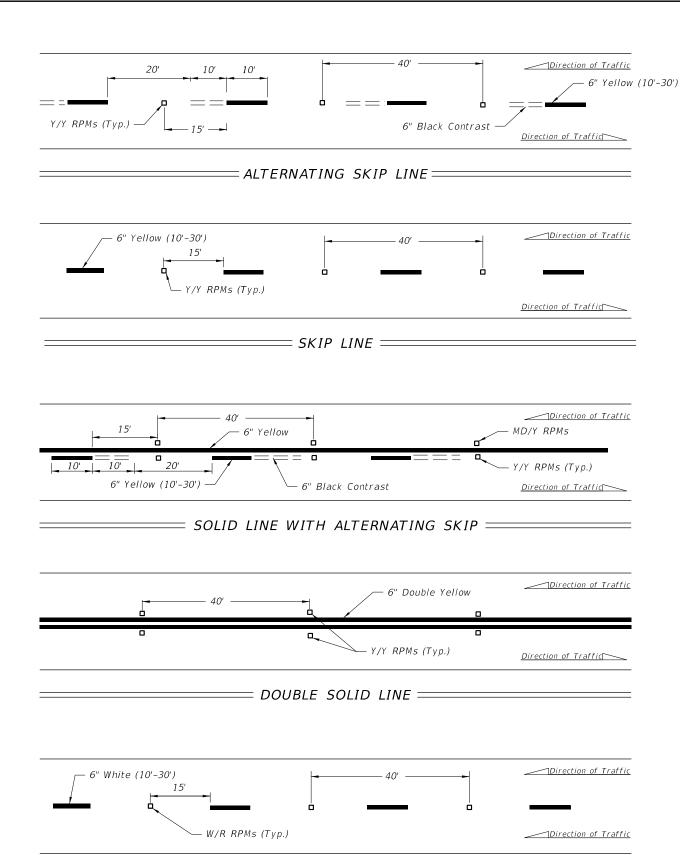
FDOT

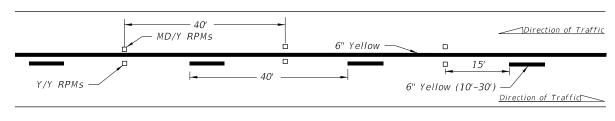
FY 2019-20 STANDARD PLANS

= DETAIL "E" ======

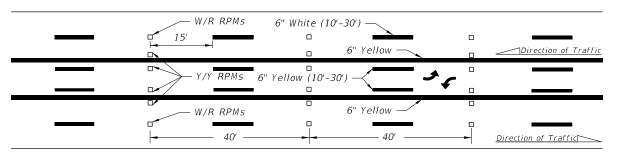
INDEX 700-120

SHEET 9 of 9

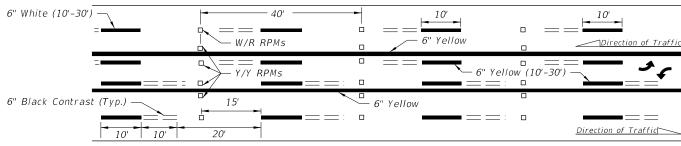




#### = SOLID LINE WITH SKIP ===



## = SKIP LINE WITH TWO-WAY LEFT TURN LANE =



## === ALTERNATING SKIP LINE WITH TWO-WAY LEFT TURN LANE ====

## **GENERAL NOTES:**

- 1. Offset all RPMs 1" from solid longitudinal lines unless otherwise noted or shown.
- 2. Spacing may be reduced for sharp curves if required.
- 3. For placement of RPMs on ramps, see Index 711-003.
- 4. Make the traffic face of the RPM the same color as the pavement marking that it is supplementing.

## LEGEND:

B/C = BACK OF CURB

EOP = EDGE OF PAVEMENT

RPM = RAISED PAVEMENT MARKER

W/R = WHITE/RED RPM

Y/Y = YELLOW/YELLOW RPM

Y/R = YELLOW/RED RPM

MD/Y = MONO-DIRECTIONAL YELLOW RPM

**REVISION** 11/01/18

DESCRIPTION:

= MULTILANE =

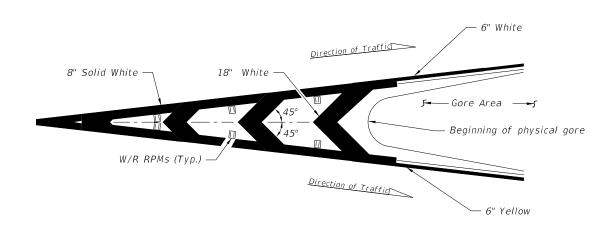
FY 2019-20

INDEX

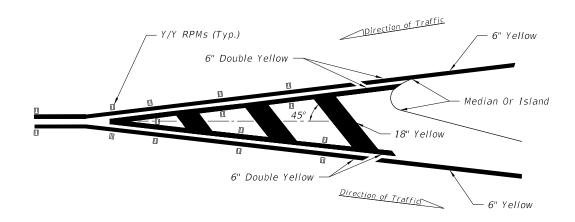
SHEET

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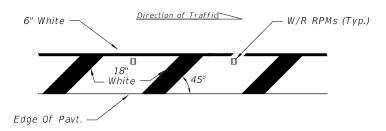
## RPM PLACEMENT AT INTERSECTIONS =



## = RPM PLACEMENT AT TRAFFIC CHANNELIZATION AT GORE ===== (Traffic Flows In Same Direction)



RPM PLACEMENT AT TRAFFIC SEPARATION =(Traffic Flows In Opposite Direction)



Right side of the roadway shown. For the left side of roadway, the pavement marking is yellow and oriented opposite hand.

## RPM PLACEMENT AT ROADSIDE CROSSHATCHING

#### *NOTE:*

1. Center the Raised Pavement Markers between chevrons and crosshatching.

#### LEGEND:

B/C = BACK OF CURB

EOP = EDGE OF PAVEMENT

RPM = RAISED PAVEMENT MARKER

W/R = WHITE/RED RPM

Y/Y = YELLOW/YELLOW RPM

Y/R = YELLOW/RED RPM

MD/Y = MONO-DIRECTIONAL YELLOW RPM

DESCRIPTION: REVISION

FDOT

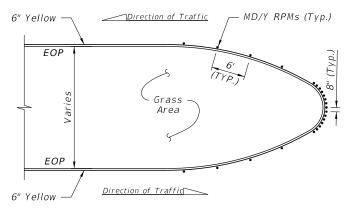
FY 2019-20 STANDARD PLANS

TYPICAL PLACEMENT OF RAISED PAVEMENT MARKERS INDEX

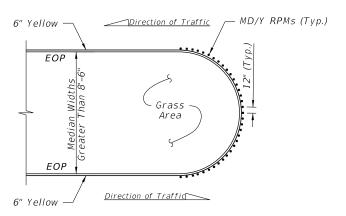
SHEET

11/01/18

DETAIL "A"

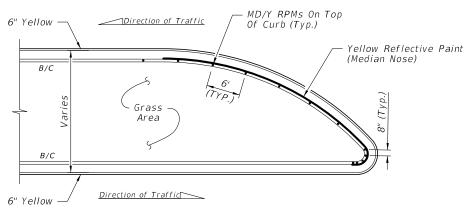


DETAIL "B'

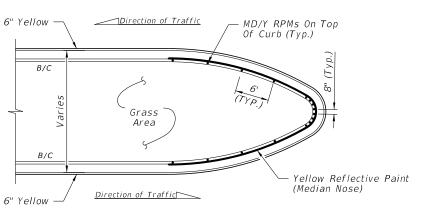


DETAIL "C"

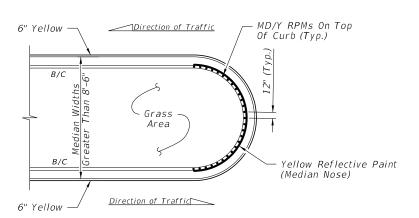
FLUSH MEDIAN OPENINGS



DETAIL "D"



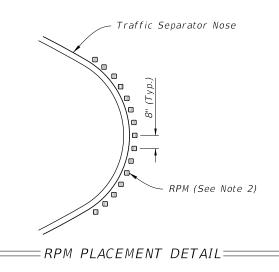
DETAIL "E"



DETAIL "F"

## TYPE "D" OR "F" CURB

RPM PLACEMENT AT MEDIAN OPENINGS = (When called for in the Plans)



POSTED SPEED LIMIT MPH	"Y" FEET		
30 OR LESS	10		
35	20		
40	20		
45	30		
50 OR MORE	40		

## LEGEND:

B/C = BACK OF CURB

EOP = EDGE OF PAVEMENT

RPM = RAISED PAVEMENT MARKER

W/R = WHITE/RED RPM

Y/Y = YELLOW/YELLOW RPM

Y/R = YELLOW/RED RPM

MD/Y = MONO-DIRECTIONAL YELLOW RPM

## NOTES:

- 1. For Type "E" Curb, install RPMs along the pavement edge marking using the same spacing shown.
- Orient traffic faces of RPMs in curb median radii to be parallel to direction of travel lanes.

LAST REVISION 11/01/18

DESCRIPTION:



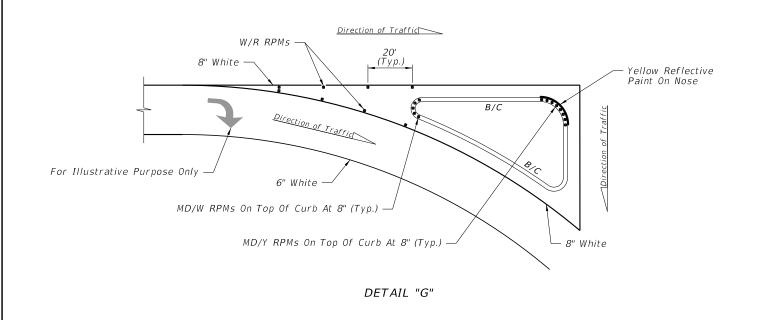
FY 2019-20 STANDARD PLANS

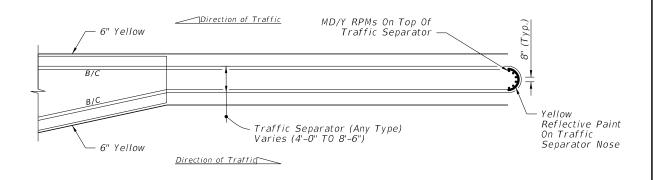
TYPICAL PLACEMENT OF RAISED PAVEMENT MARKERS

INDEX

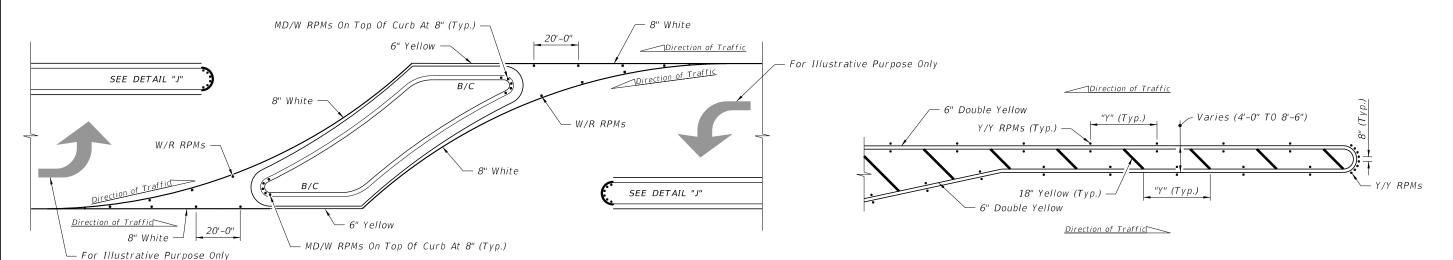
SHEET

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DETAIL "J"



DETAIL "H"

DETAIL "K"

## RPM PLACEMENT AT ISLANDS = (When called for in the Plans)

## RPM PLACEMENT AT TRAFFIC SEPARATORS $\equiv$ (When called for in the Plans)

#### POSTED SPEED LIMIT MPH FÉET 30 OR LESS 10 35 20 40 20 45 30 50 OR MORE 40

## NOTES:

- 1. For Type "E" Curb install RPMs along the pavement edge marking using the same spacing shown.
- 2. Orient traffic faces of RPMs in median radii to be parallel to direction of travel lanes.

## LEGEND:

B/C = BACK OF CURB

EOP = EDGE OF PAVEMENT

RPM = RAISED PAVEMENT MARKER

W/R = WHITE/RED RPM

Y/Y = YELLOW/YELLOW RPM

Y/R = YELLOW/RED RPM

MD/Y = MONO-DIRECTIONAL YELLOW RPM

MD/W = MONO-DIRECTIONALWHITE RPM

**REVISION** 11/01/18

DESCRIPTION:

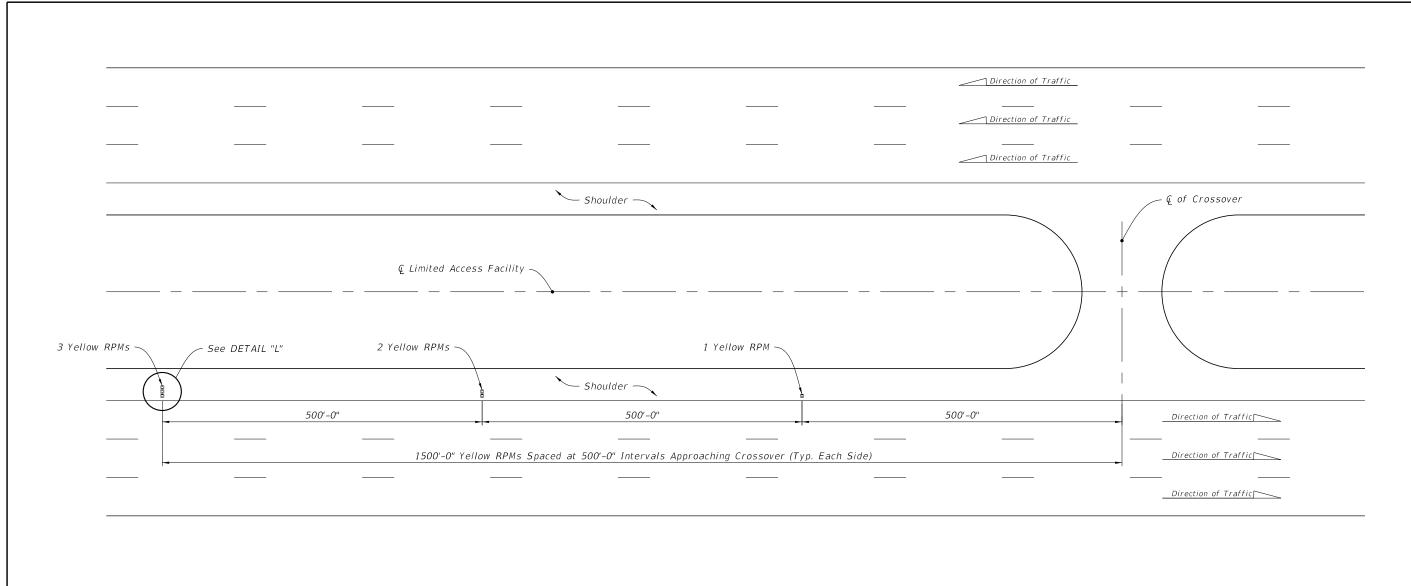
FDOT

FY 2019-20 STANDARD PLANS

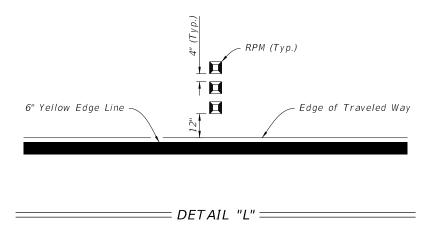
TYPICAL PLACEMENT OF RAISED PAVEMENT MARKERS INDEX

SHEET

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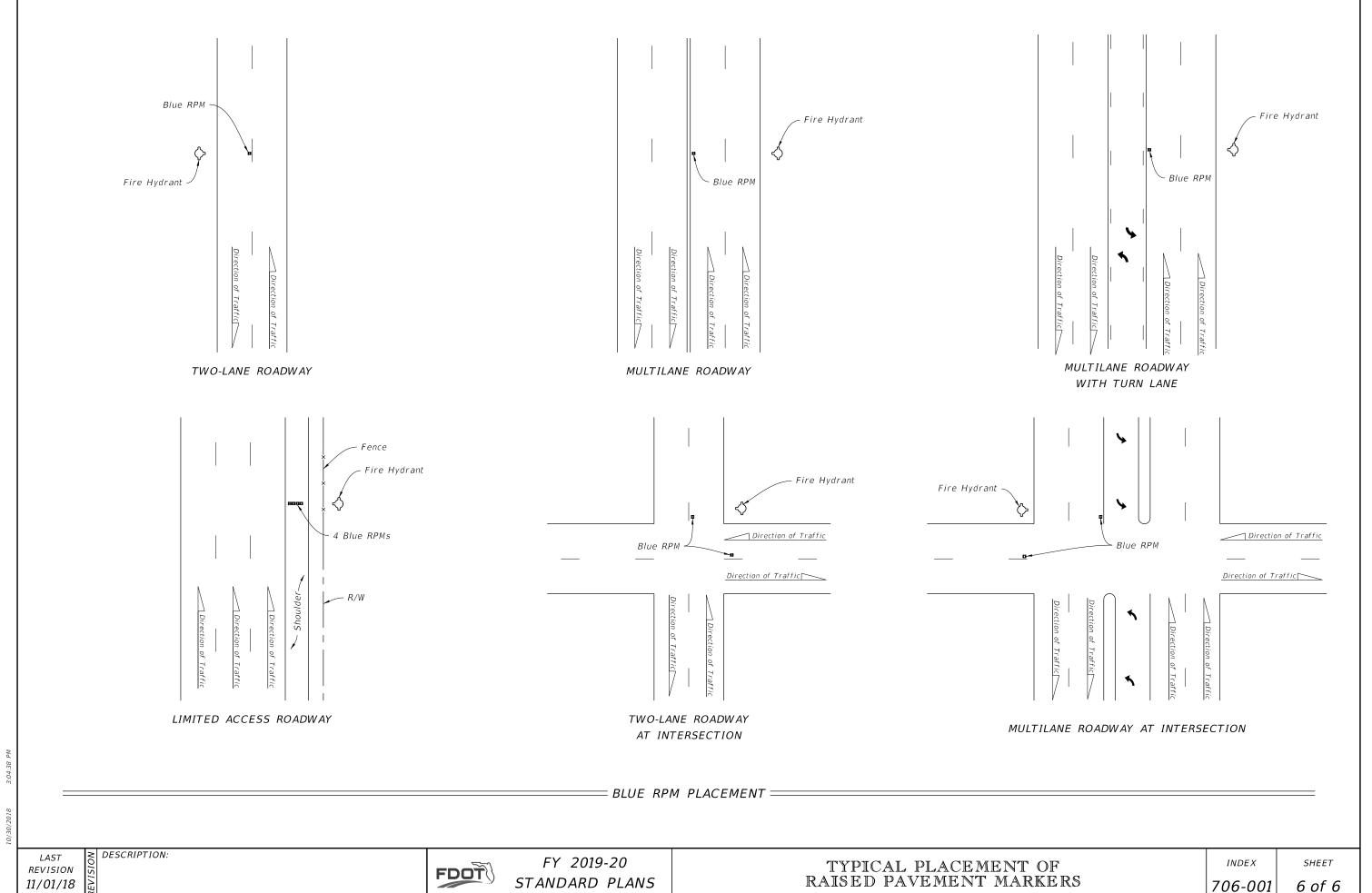
=== RPM PLACEMENT FOR CROSSOVERS ON LIMITED ACCESS ROADWAYS ======

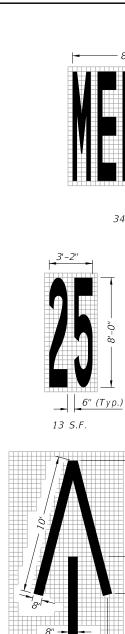


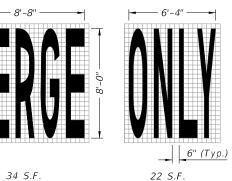
REVISION 11/01/18

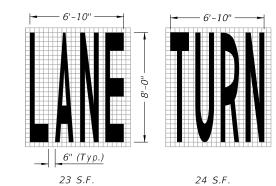
≥ DESCRIPTION:

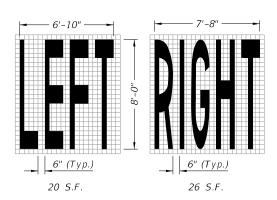
FDOT





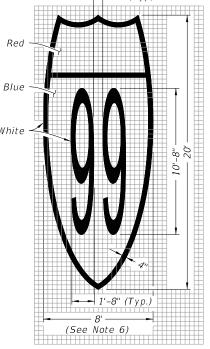






10'-10"

43 S.F.

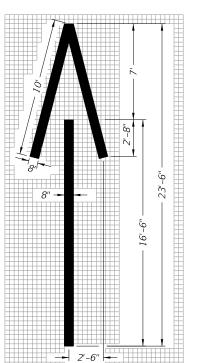


→ | <del>-</del> 6" (Typ) (See Note 6) Route Shield for Arterials

Route Shield for Limited Access Roadways (Interstate Route Shield Shown; U.S. and State Route Shield Similar)

and Collectors (Interstate Route Shield Shown; U.S. and State Route Shield Similar) 72 S.F.

128 S.F.

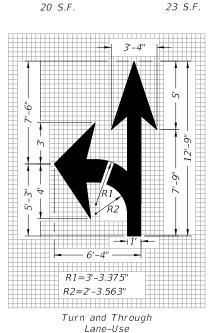


Wrong-Way Arrow

24 S.F.

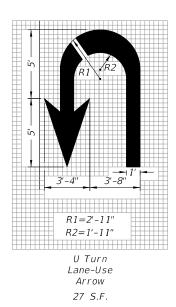
DESCRIPTION:

20 S.F.



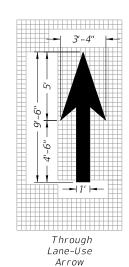
Arrow

6" (Typ.)



6" (Typ.)

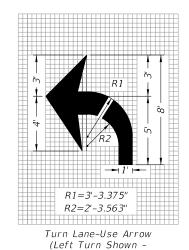
22 S.F.



12 S.F.

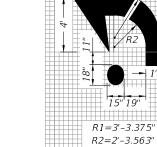
3. Dimensions are within 1" ±.

20 S.F.

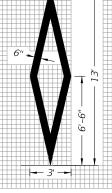


Right Turn Similar)

17 S.F.



R2=2'-3.563" Roundabout Approach Arrow 19 S.F.



Preferential Lane Symbol 11 S.F.

#### 29 S.F. NOTES FOR PAVEMENT MESSAGES:

1. When an arrow and a pavement message are used together, locate the arrow a distance of "S" downstream from the pavement message. Measure the distance from the base of the arrow to the base of the pavement message. See the Pavement Message Spacing Table for "S" value.

2. Place all pavement messages 25' back from the stop line.

- 4. All grids are 4" x 4".
  - 5. All pavement messages must be white except route shields.
  - 6. Increase width of route shield for routes with three digits.

PAVEMENT MESSAGE SPACING TABLE					
Posted Speed (mph)	Distance "S" (feet)				
≤ 25	40				
30 - 35	56				
40 - 45	72				
≥ 50	88				

PAVEMENT MESSAGE AND ARROW DETAILS

## GENERAL NOTE:

1. See Index 509-070 for pavement markings at railroad crossings.

**REVISION** 11/01/18

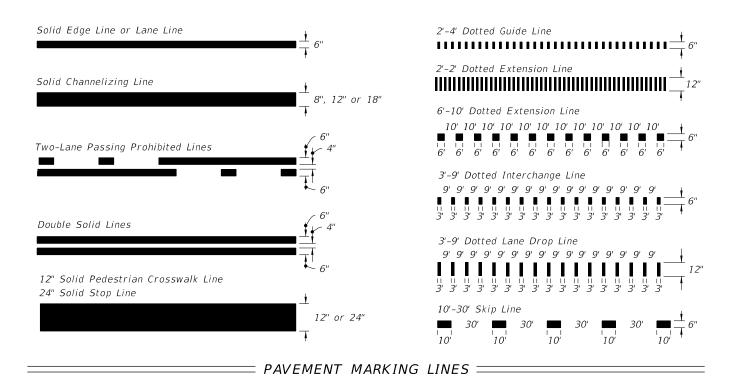
FDOT

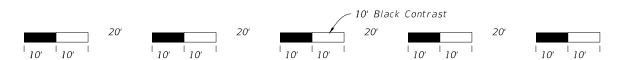
FY 2019-20 STANDARD PLANS

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SHEET

1 of 13

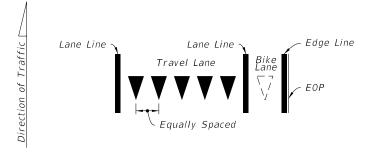




10' White Skip With 10' Black Contrast and 20' Gaps

## = CONTRAST MARKINGS WITH ALTERNATING SKIP PATTERN ======

(10'-30' Skip Line Shown, Dotted Lines Similar)



Yield Lines consist of five - 18" X 27" white triangles which face traffic. Equally space triangles within traffic lane. When a bike lane is present, add one additional triangle in the center of the bike lane.

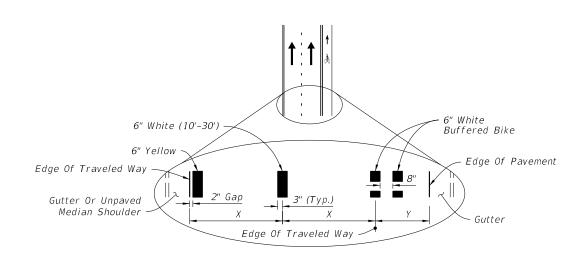
= YIELD LINES =

10/30/2018 3:0

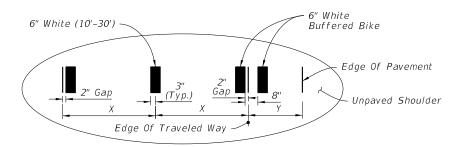
LAST REVISION 11/01/18

DESCRIPTION:





## CURB AND GUTTER

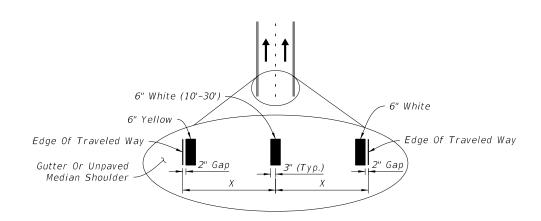


## FLUSH SHOULDER

X = LANE WIDTH (FT.)

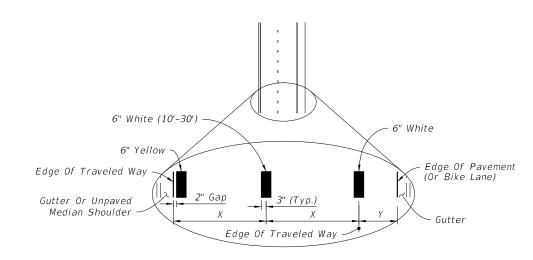
Y = BUFFERED BIKE LANE WIDTH (FT.)

## = STRIPING FOR BUFFERED BIKE LANE =

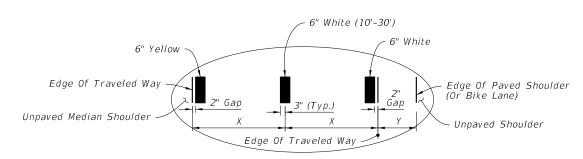


X = LANE WIDTH (FT.)

= STRIPING WITH NO SHOULDER OR BIKE LANE ==



## CURB AND GUTTER



## FLUSH SHOULDER

X = LANE WIDTH (FT.)

Y = PAVED SHOULDER / BIKE LANE

## = STRIPING WITH SHOULDER OR NON-BUFFERED BIKE LANE ==

## NOTES:

- 1. Lane widths (X) may not be same for each lane in the section.
- 2. For placement of RPMs, see Index 706-001.

## PLACEMENT OF LONGITUDINAL PAVEMENT MARKINGS

REVISION 11/01/18

DESCRIPTION:

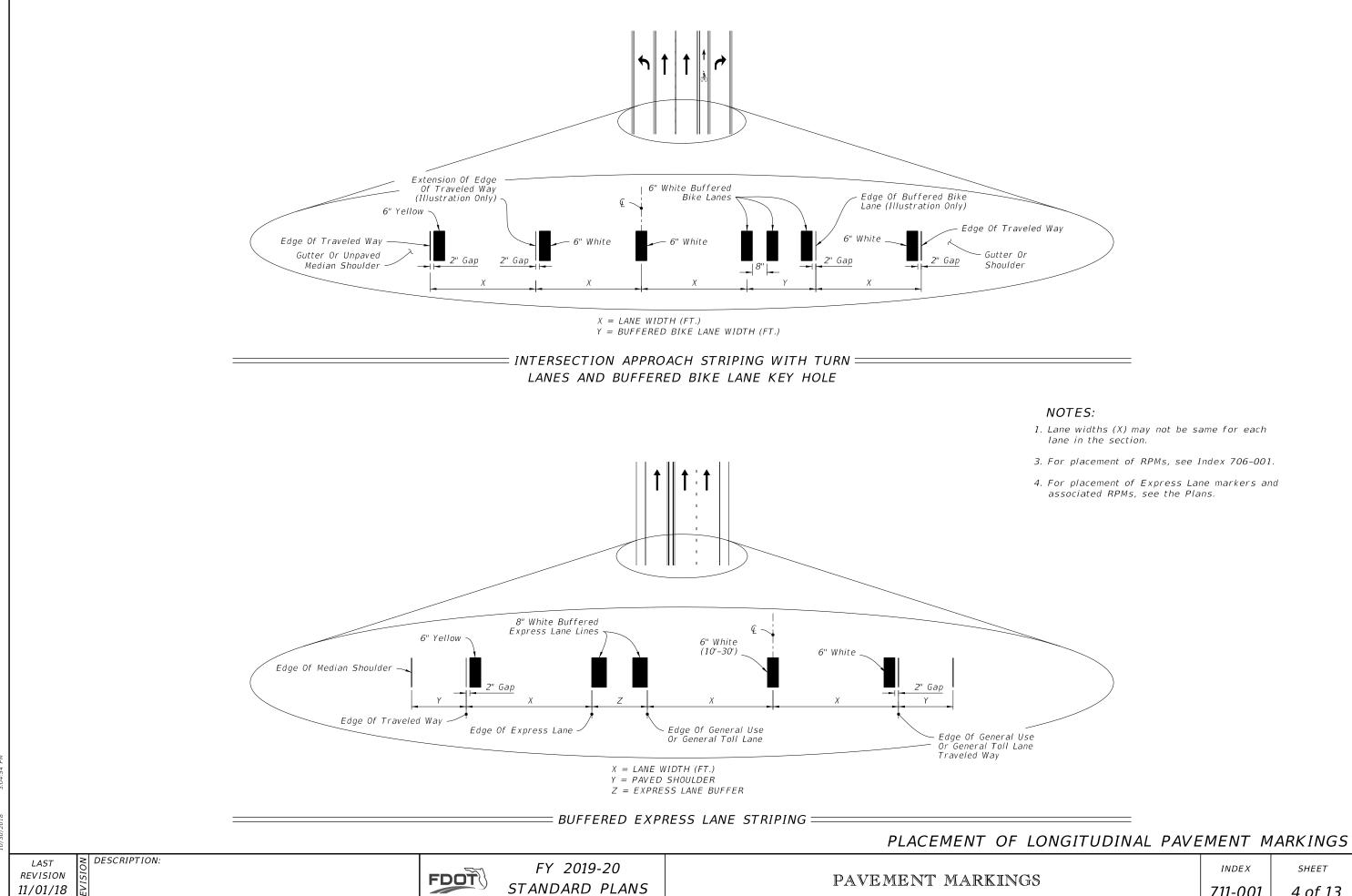
FDOT

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SHEET

PAVEMENT MARKINGS

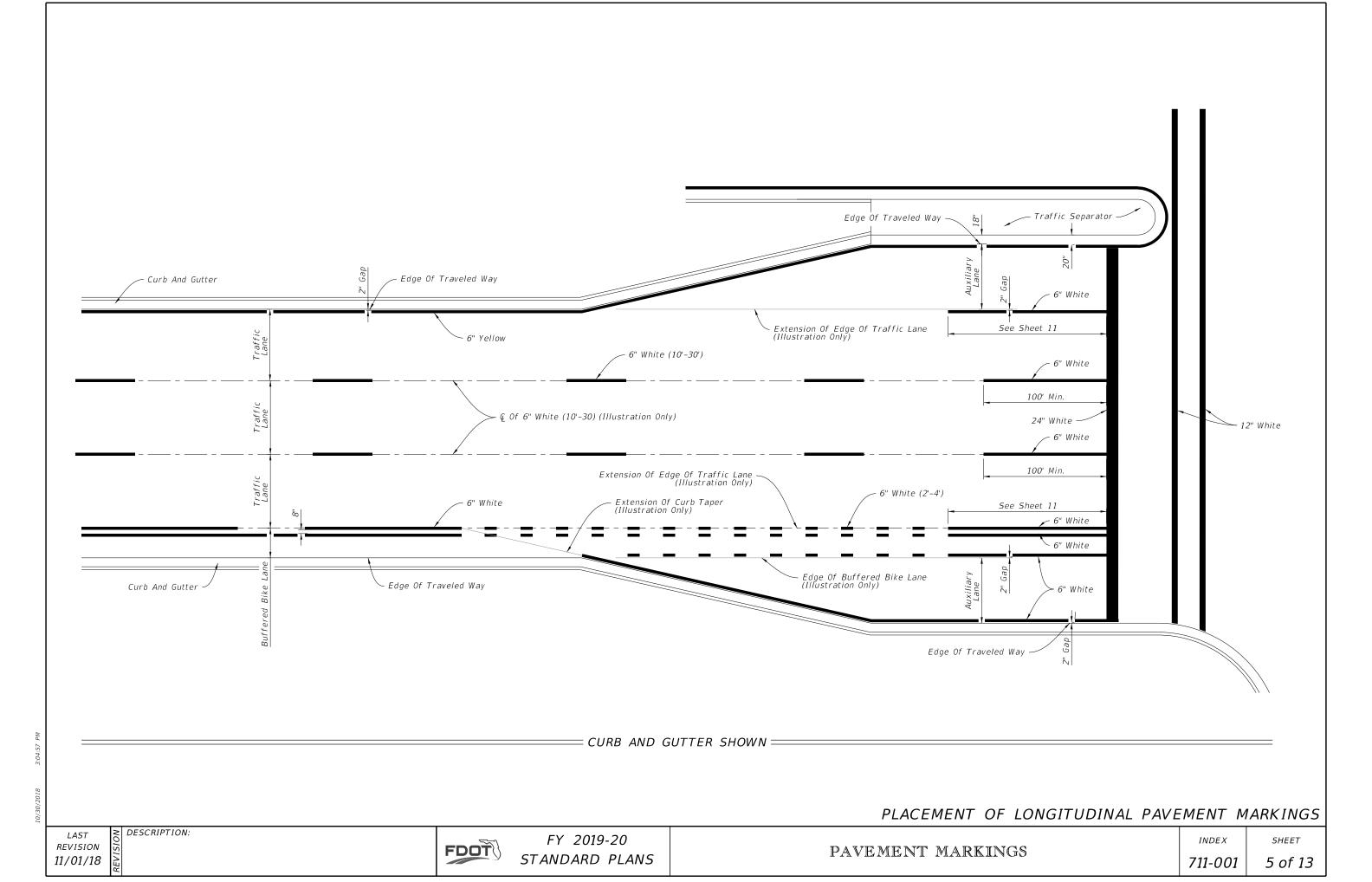


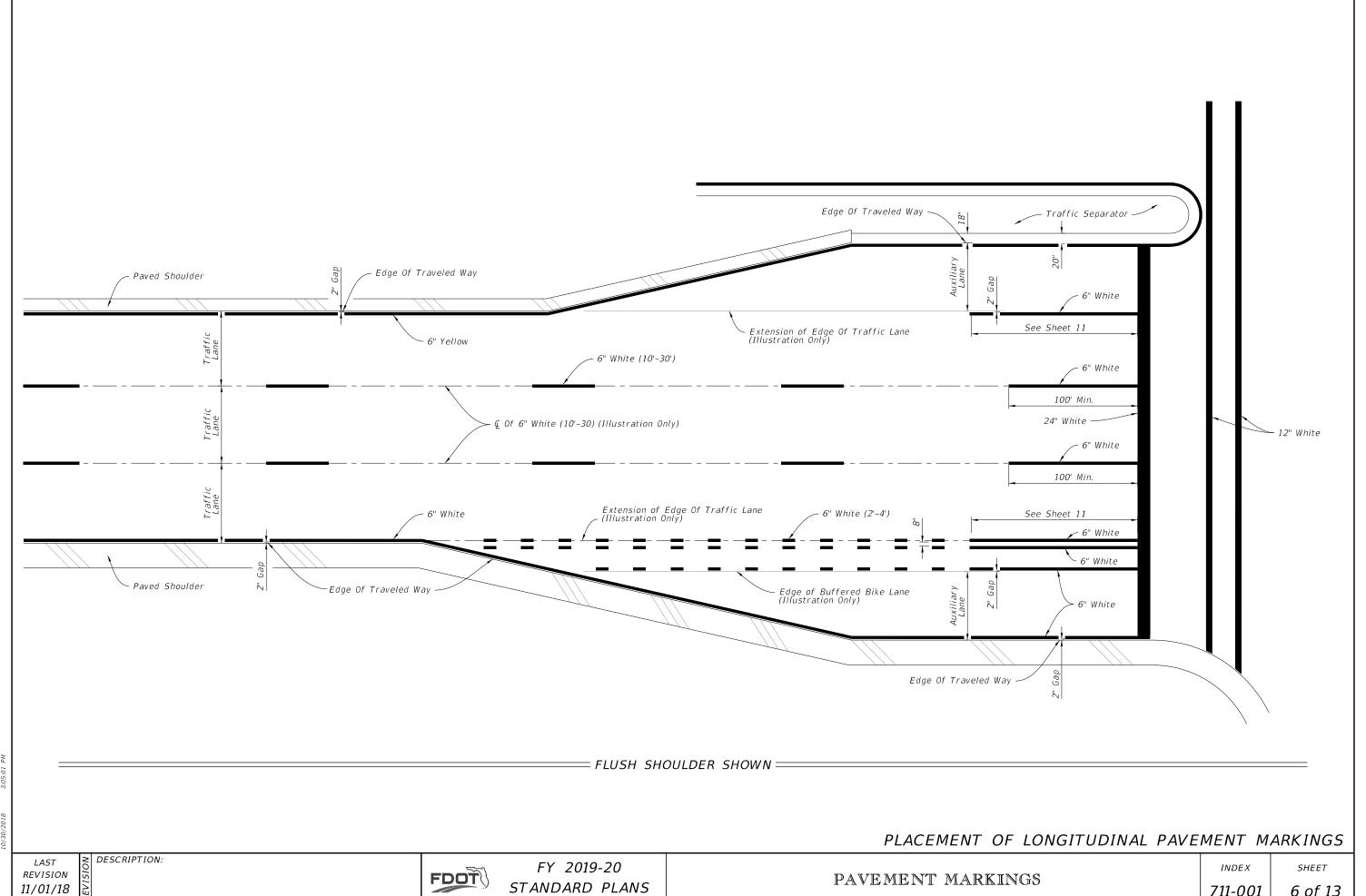
FDOT

STANDARD PLANS

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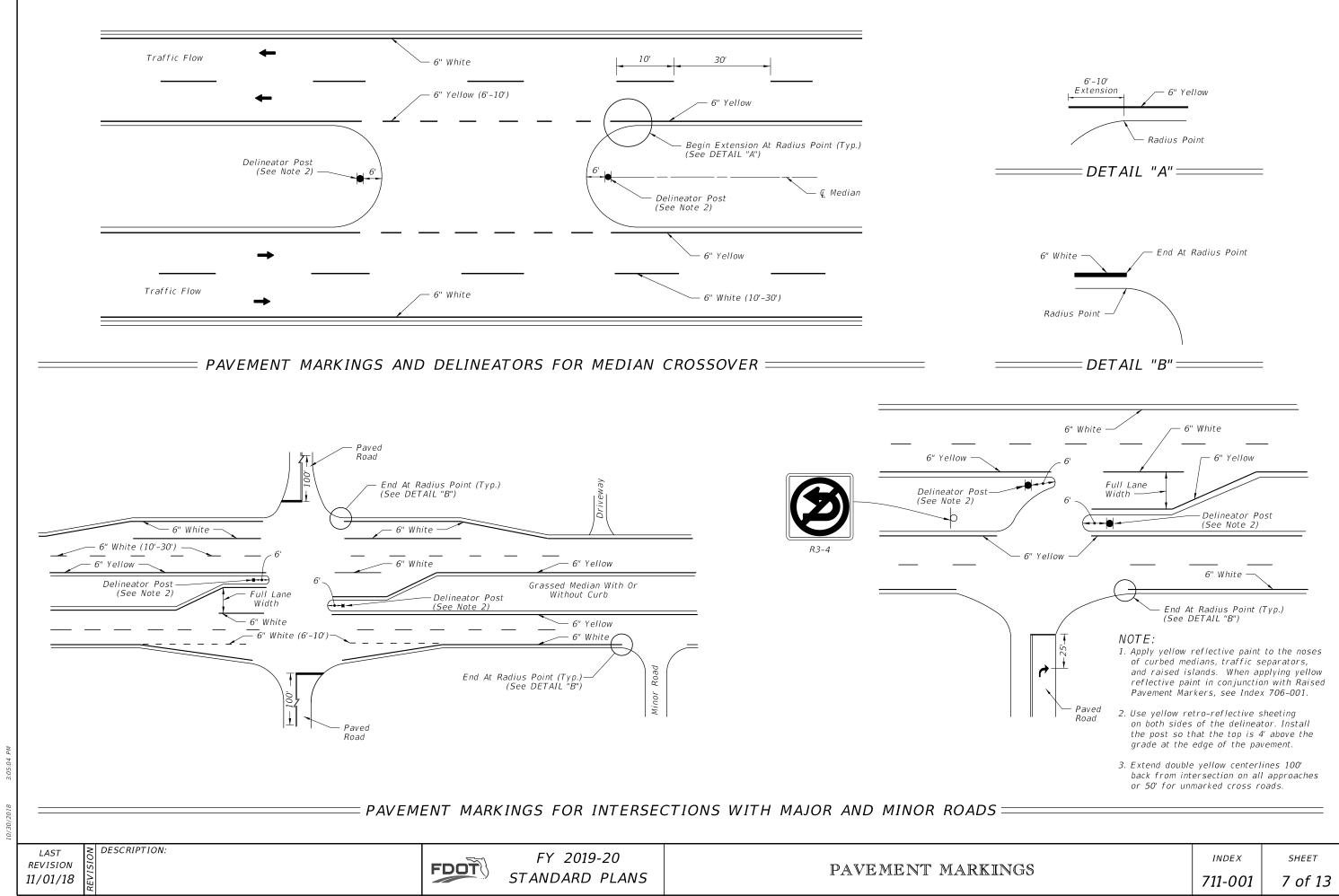


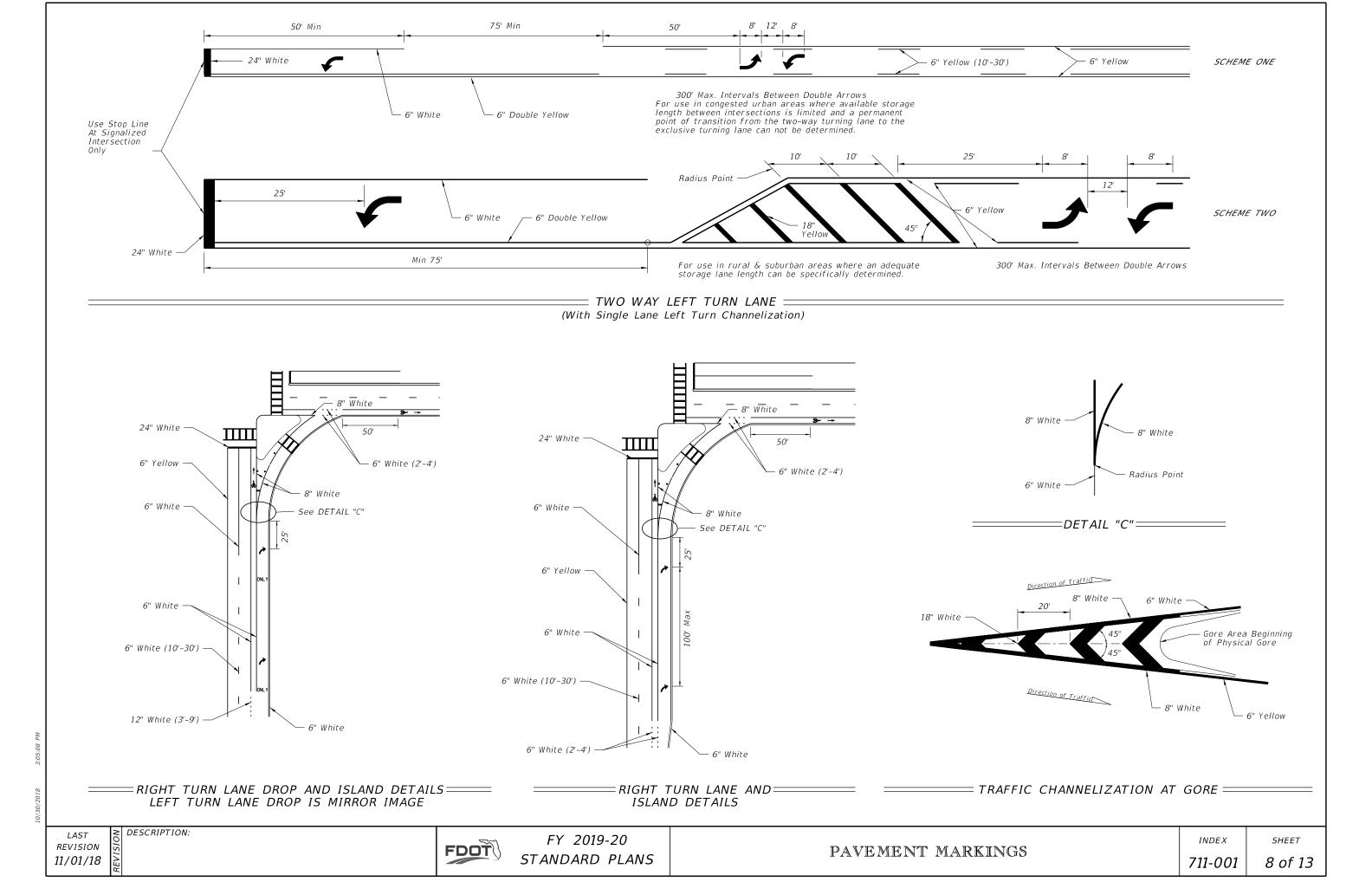


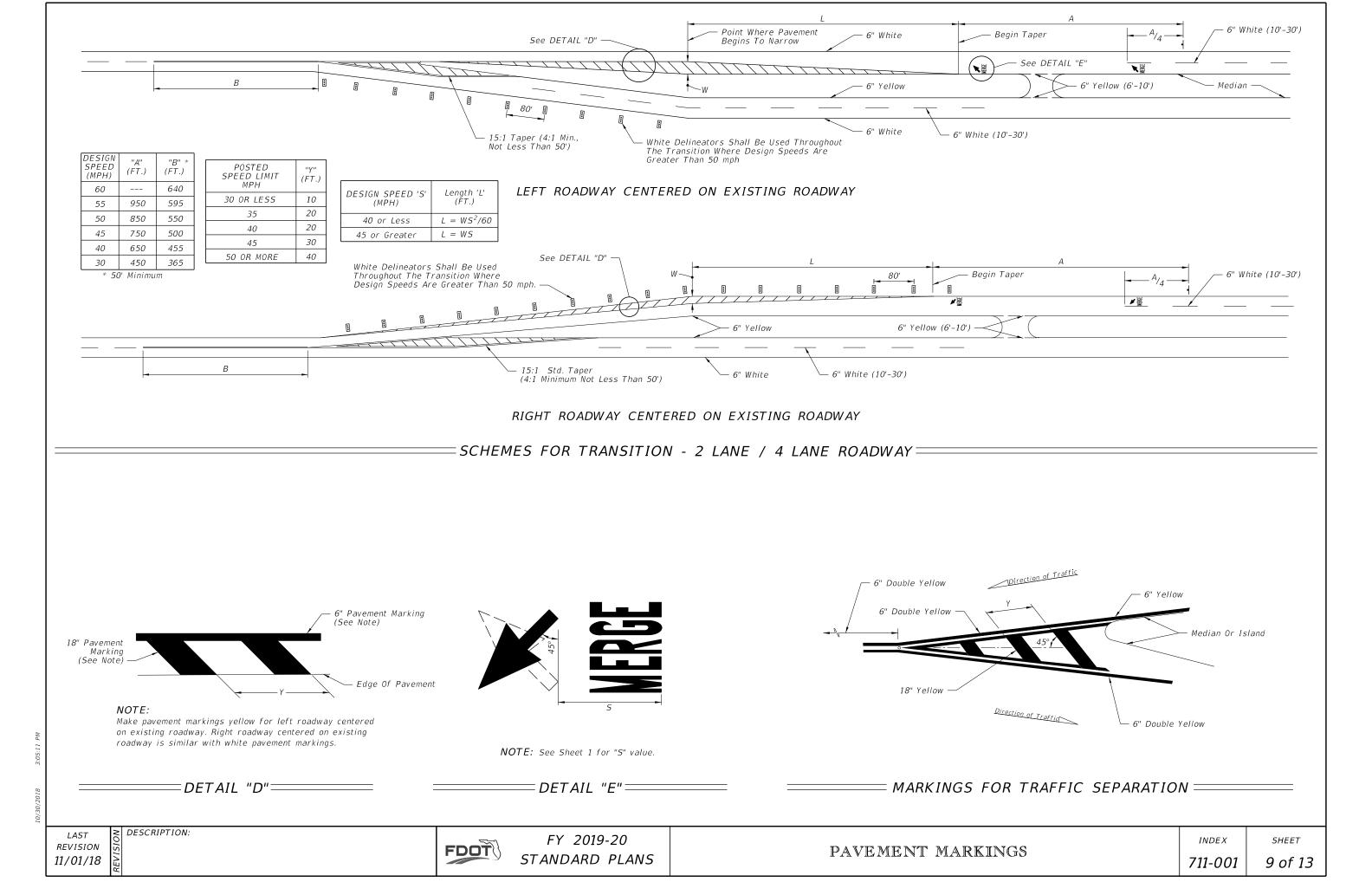
STANDARD PLANS

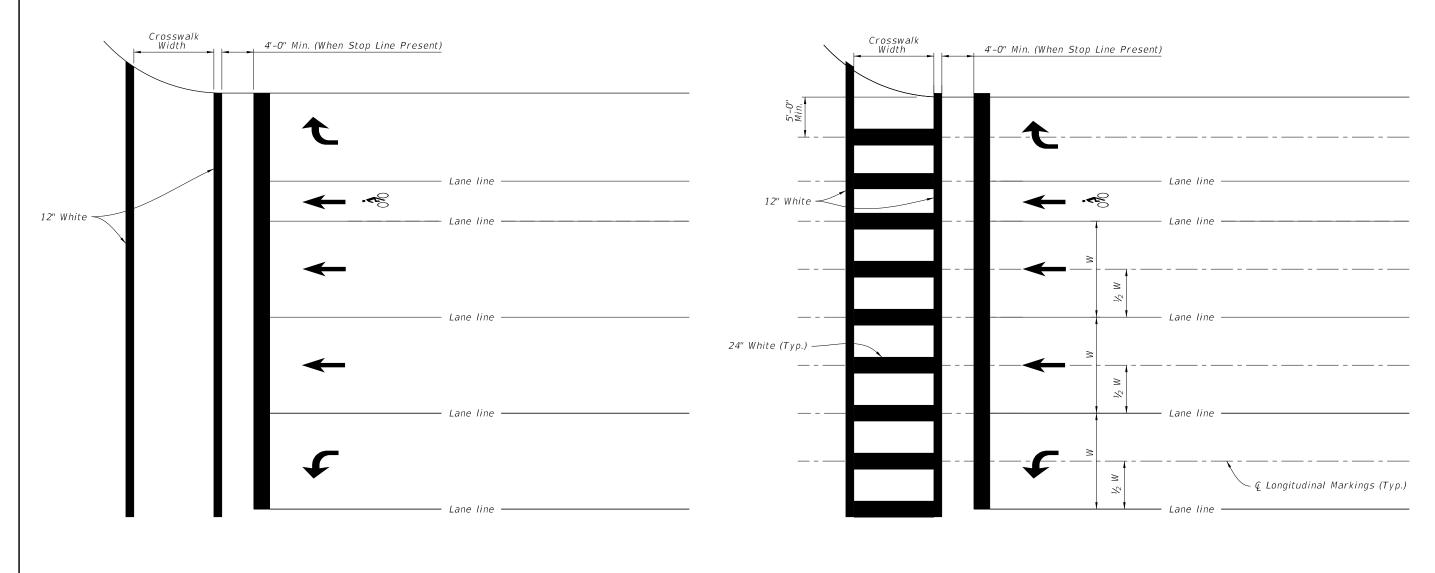
711-001

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STANDARD CROSSWALK DETAILS

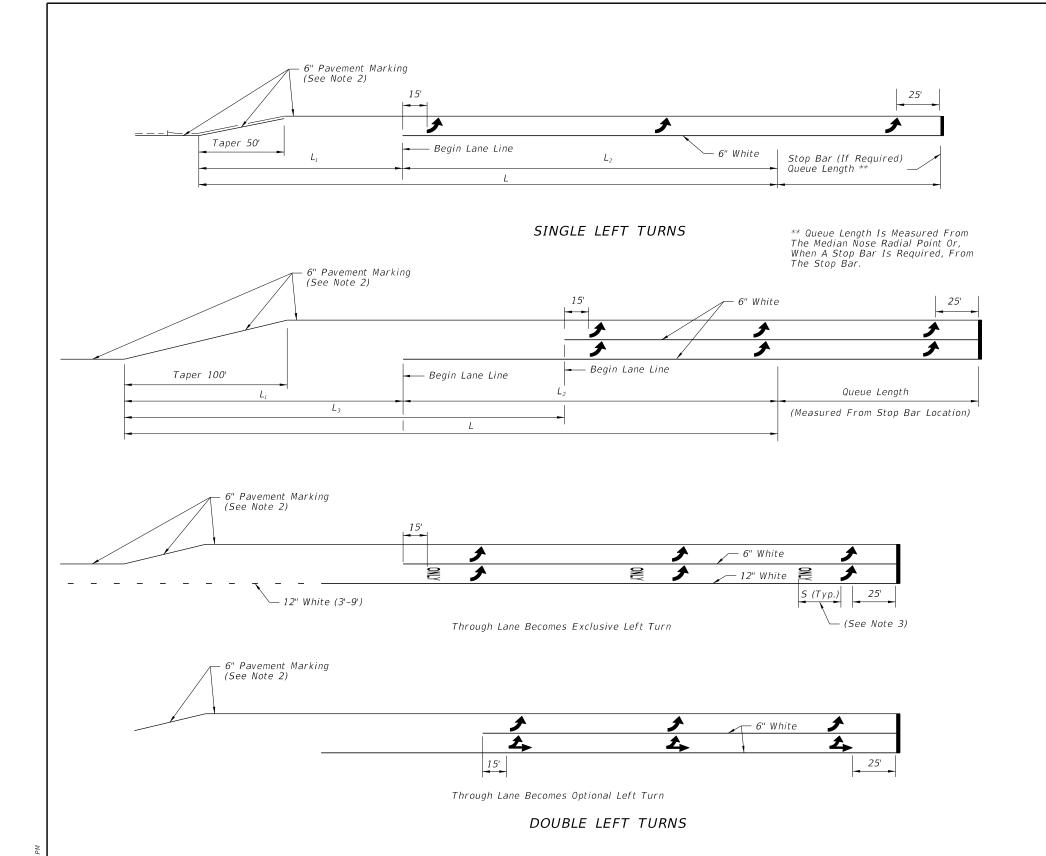
=SPECIAL EMPHASIS CROSSWALK DETAILS ====

## NOTES:

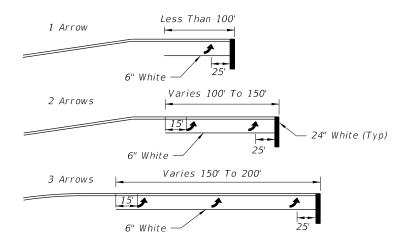
- 1. For crosswalk width, exceed width of the adjacent sidewalk, but do not make width less than 6' for intersection crosswalks and 10' for midblock crosswalks. Measure width from the inside of the transverse crosswalk markings.
- 2. When the Special Emphasis Crosswalk is not perpendicular to the lane lines, make the longitudinal markings parallel to the lane lines.
- 3. Refer to Index 522-002 when Curb Ramps are present.

REVISION 11/01/18

DESCRIPTION:



	TURN LANES • CURBED AND UNCURBED MEDIANS							
		URBAN CONDITIONS			RURAL CONDITIONS			
Design Speed (mph)	Clearance Distance	Brake To Stop Distance	Total Decel. Distance	Clearance Distance	Brake To Stop Distance	Total Decel. Distance	Clearance Distance	
	L,	L ₂	L	L ₃	L ₂	L	L ₃	
35	70'	75'	145'	110'	— –		— –	
40	80'	7 <i>5</i> ′	155'	120'				
45	85'	100'	185'	135'				
50	105'	135'	240'	160'	185'	290'	160'	
55	125'				225'	350'	195'	
60	145'				260'	405'	230'	
65	170'				290'	460'	270'	



Arrow should be evenly spaced between first and last arrow. Turn lanes longer than 200' add one arrow for each 100' additional length.

## ARROW SPACING

## NOTES:

- 1. This Index also applies to right turn lanes.
- 2. Make pavement marking yellow for left-turn lanes and white for right-turn lanes.
- 3. See Sheet 1 for "S" value.

= TURN LANE MARKINGS =

**REVISION** 11/01/18

DESCRIPTION:

FDOT

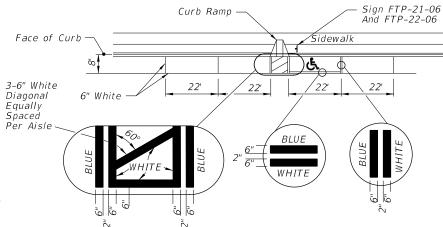
FY 2019-20 STANDARD PLANS

PAVEMENT MARKINGS

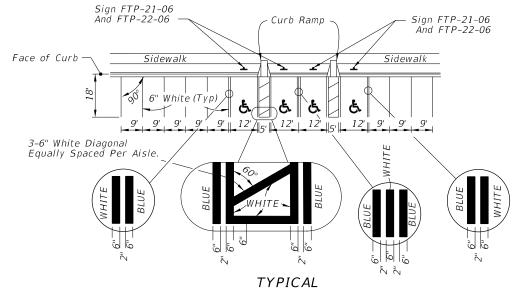
INDEX 711-001

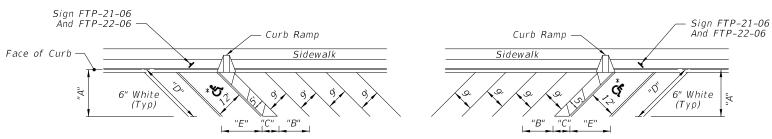
SHEET

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- 1. Dimensions are to the centerline
- 2. An Access Aisle is required for each accessible space when angle parking is used.
- 3. Criteria for pavement markings only, not public sidewalk curb ramp locations. For ramp locations refer to plans.
- 4. Tint blue pavement markings to match color 15180 of Federal Standards 595a.
- 5. Mount FTP-22-06 sign below the FTP-21-06 sign.





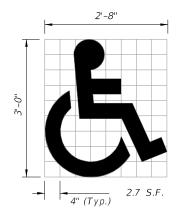
FORWARD-IN PARKING

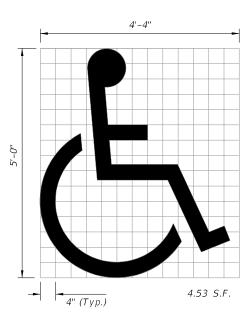
REVERSE-IN PARKING

*FOR ACCESSIBLE MARKINGS - SEE ABOVE

	"DIMENSIONS"					
6 ≽	"A"	"B"	"C"	"D"	"E"	
45°	19'-1"	12'-9"	7'-0"	27'-0"	17'-0"	
60°	20'-1"	10'-5"	5'-9"	23'-2"	13'-10"	

PAVEMENT MARKING FOR PARKING=





b Use of pavement symbol in accessible parking spaces is optional, when used the symbol shall be 3' or 5' high and white in color.

=UNIVERSAL SYMBOL OF ACCESSIBILITY=

LAST **REVISION** 11/01/18

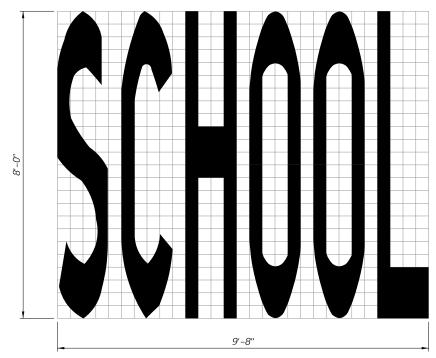
DESCRIPTION:

FDOT

FY 2019-20 STANDARD PLANS

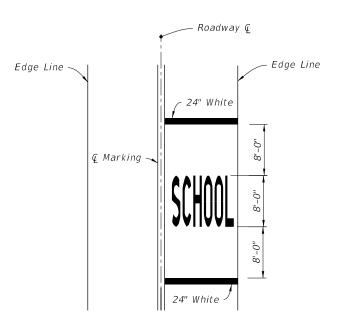
PAVEMENT MARKINGS

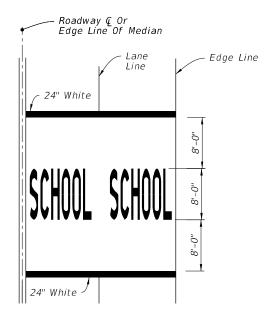
INDEX

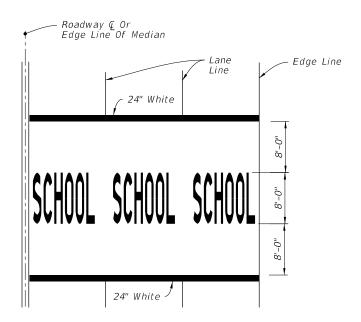


SCHOOL PAVEMENT MARKING

- 1. All grids are 4" x 4".
- 2. Pavement Marking Should Not Extend Into Opposing Lane.
- 3. Center School Pavement Marking in lane.







SINGLE-LANE APPROACH

TWO-LANE APPROACH

MULTI-LANE APPROACH (Three or More)

= MARKINGS FOR SCHOOL ZONES =

REVISION 11/01/18

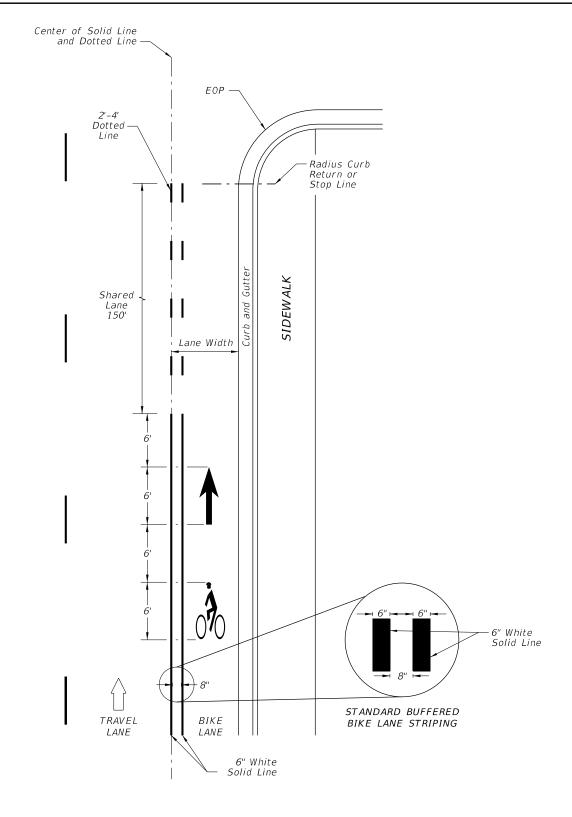
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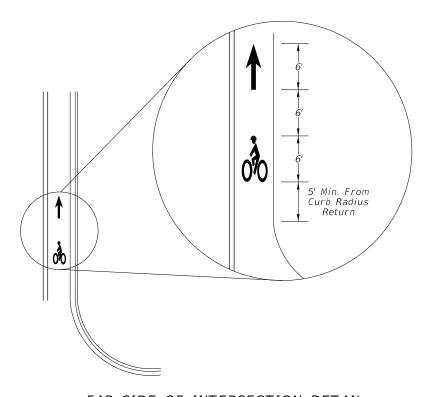
FDOT

FY 2019-20 STANDARD PLANS

PAVEMENT MARKINGS

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FAR SIDE OF INTERSECTION DETAIL

APPROACH TO INTERSECTIONS DETAILS

= BUFFERED BIKE LANES =

REVISION 11/01/17

≥ DESCRIPTION:

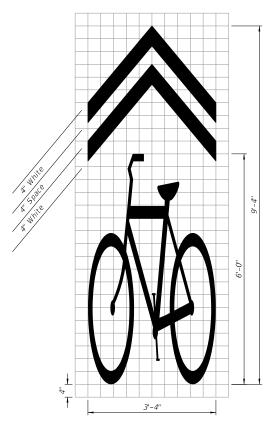
FDOT

FY 2019-20 STANDARD PLANS

BICYCLE MARKINGS

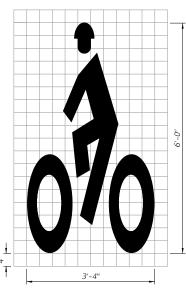
INDEX 711-002

8.1 S.F.



Shared Lane Marking (SLM)

6.3 S.F.

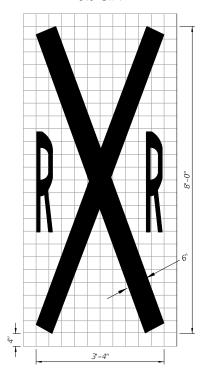


Helmeted Bicyclist Symbol

4.2 S.F.

Bike Lane Arrow

9.0 S.F.



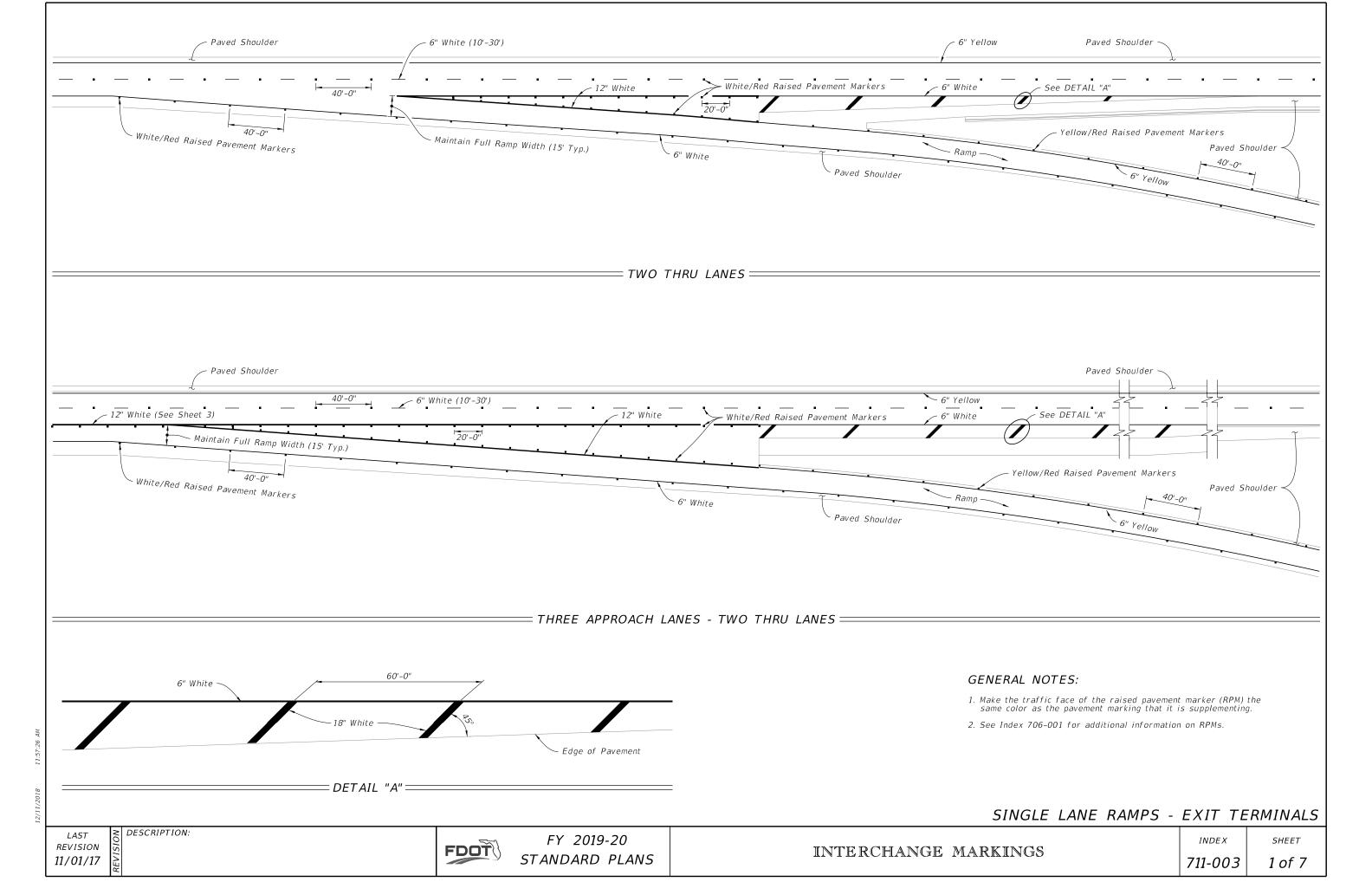
Railroad Crossing (For Shared Use Path Only)

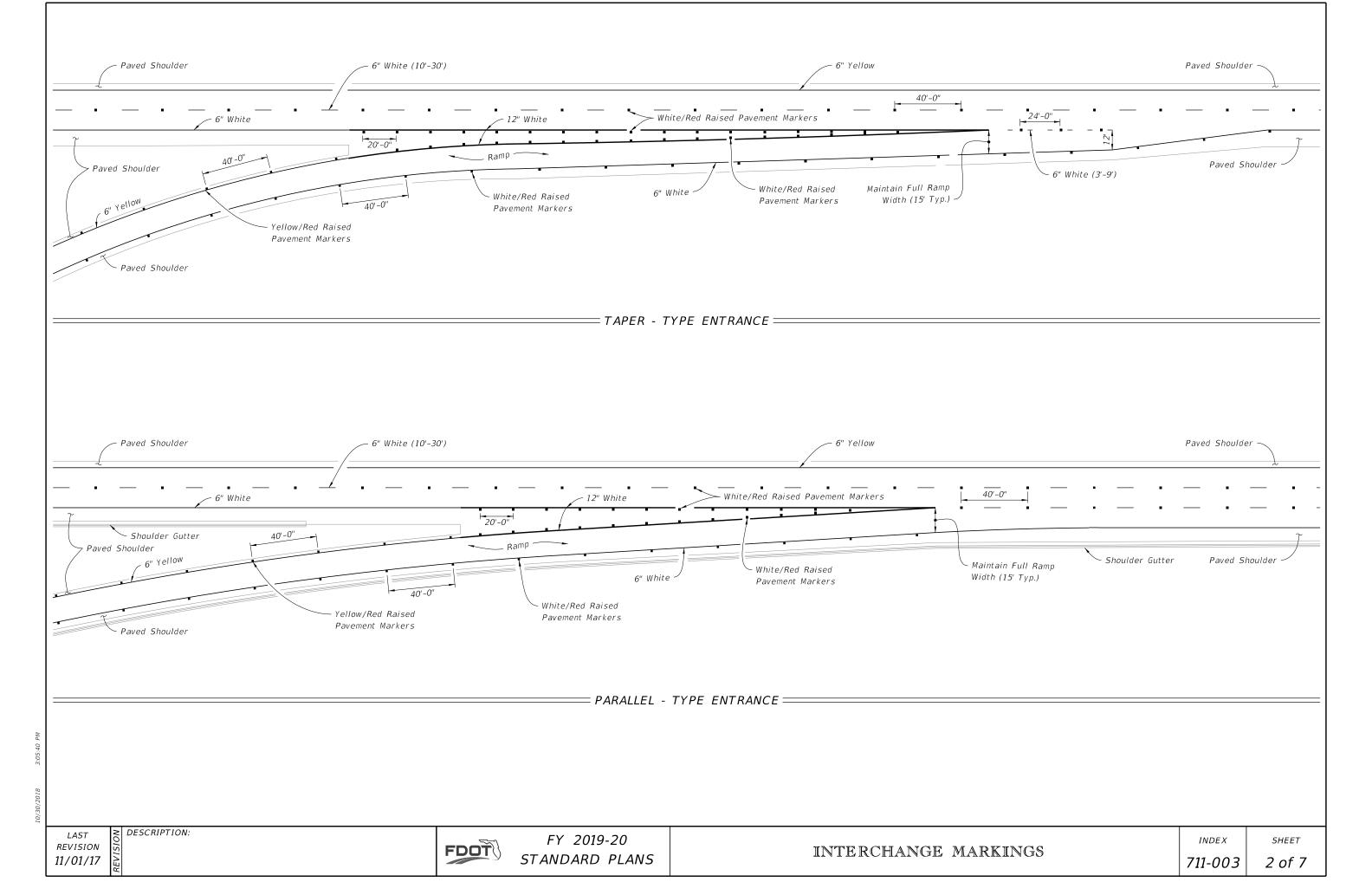
## NOTES:

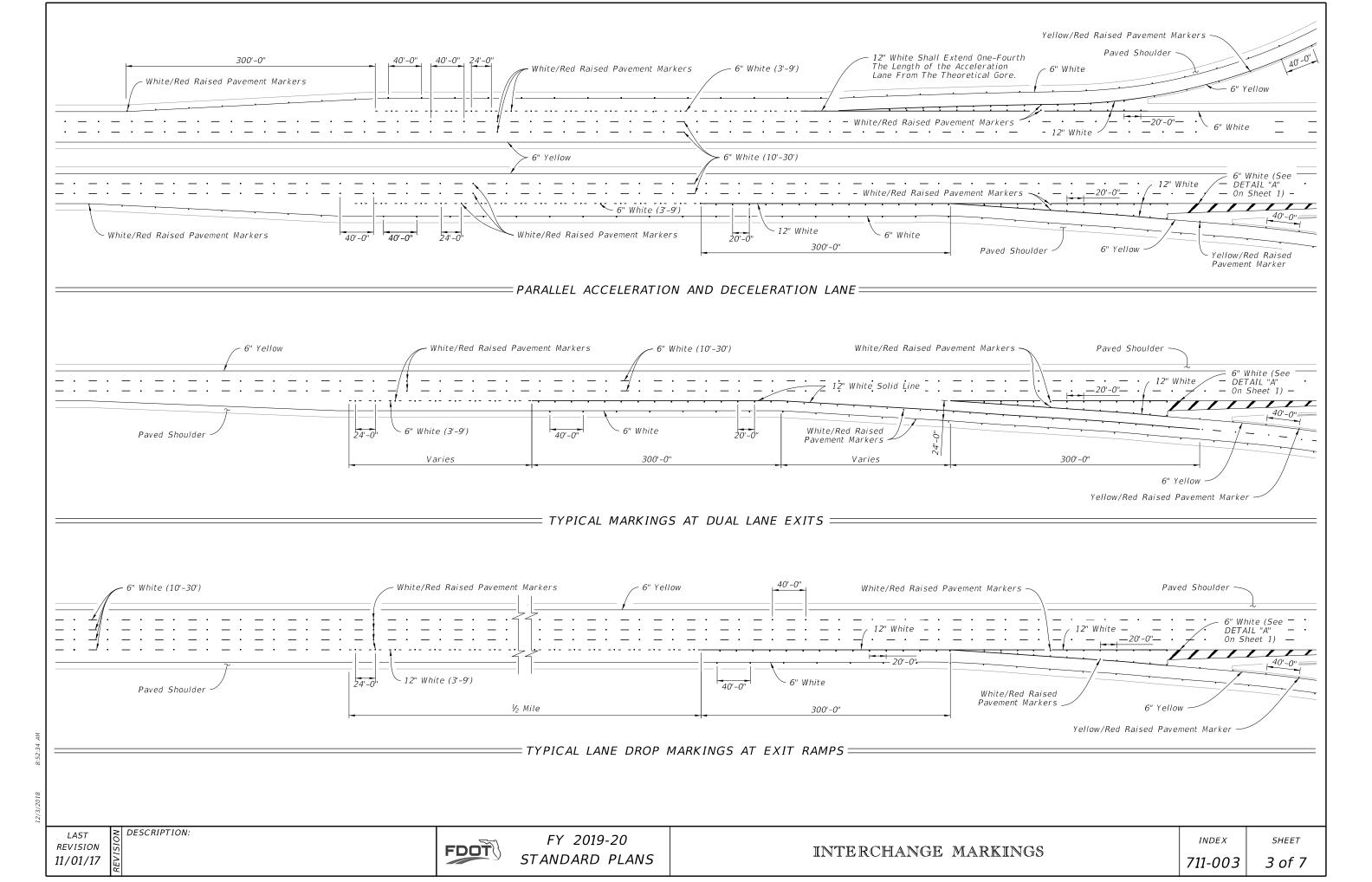
- 1. All bicycle markings and pavement messages shall be White.
- 2. All bicycle markings shall be preformed thermoplastic.
- 3. All grids are 4" x 4".

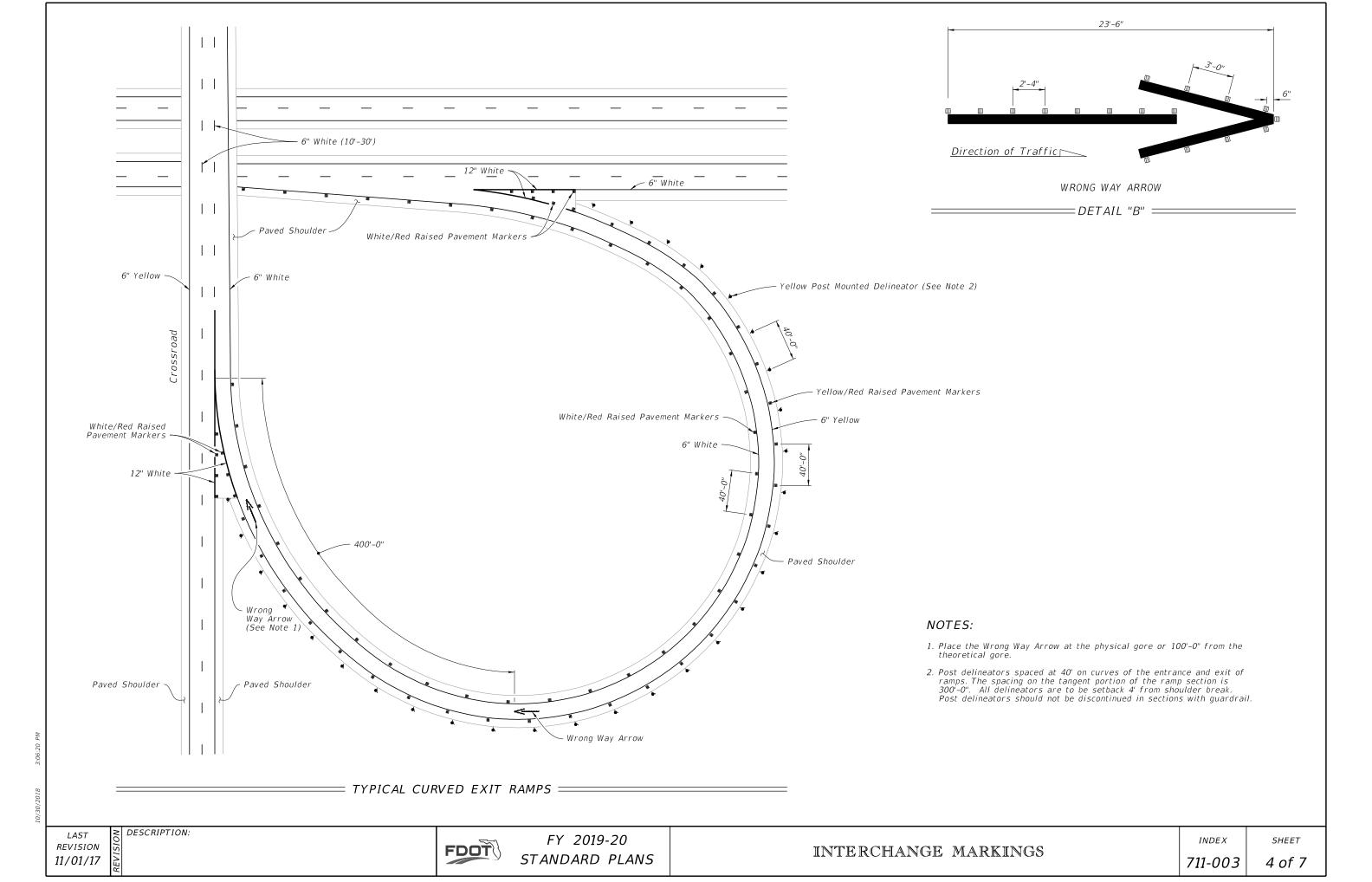
STANDARD PAVEMENT MARKING MESSAGE LAYOUTS

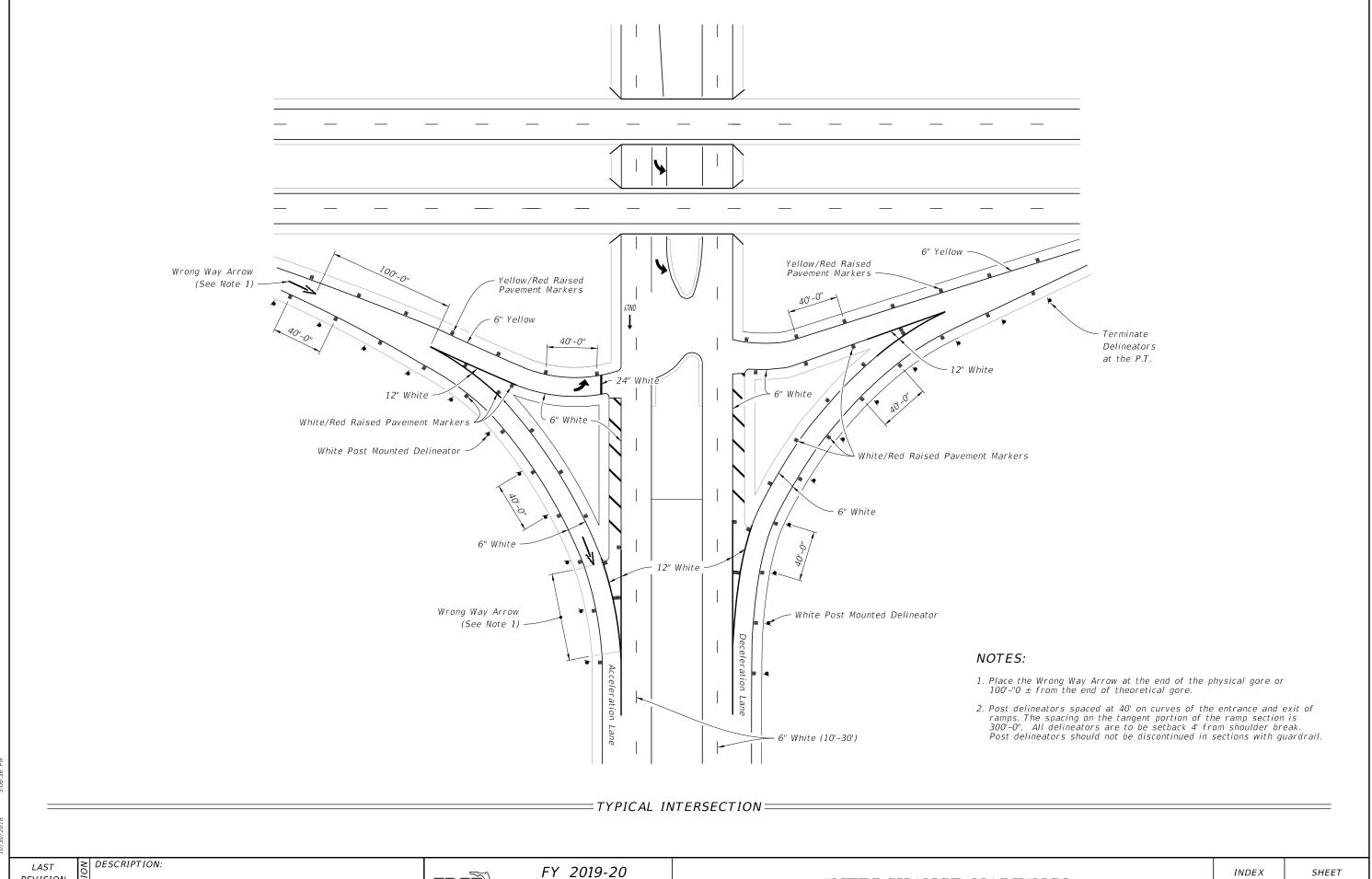
≥ DESCRIPTION:



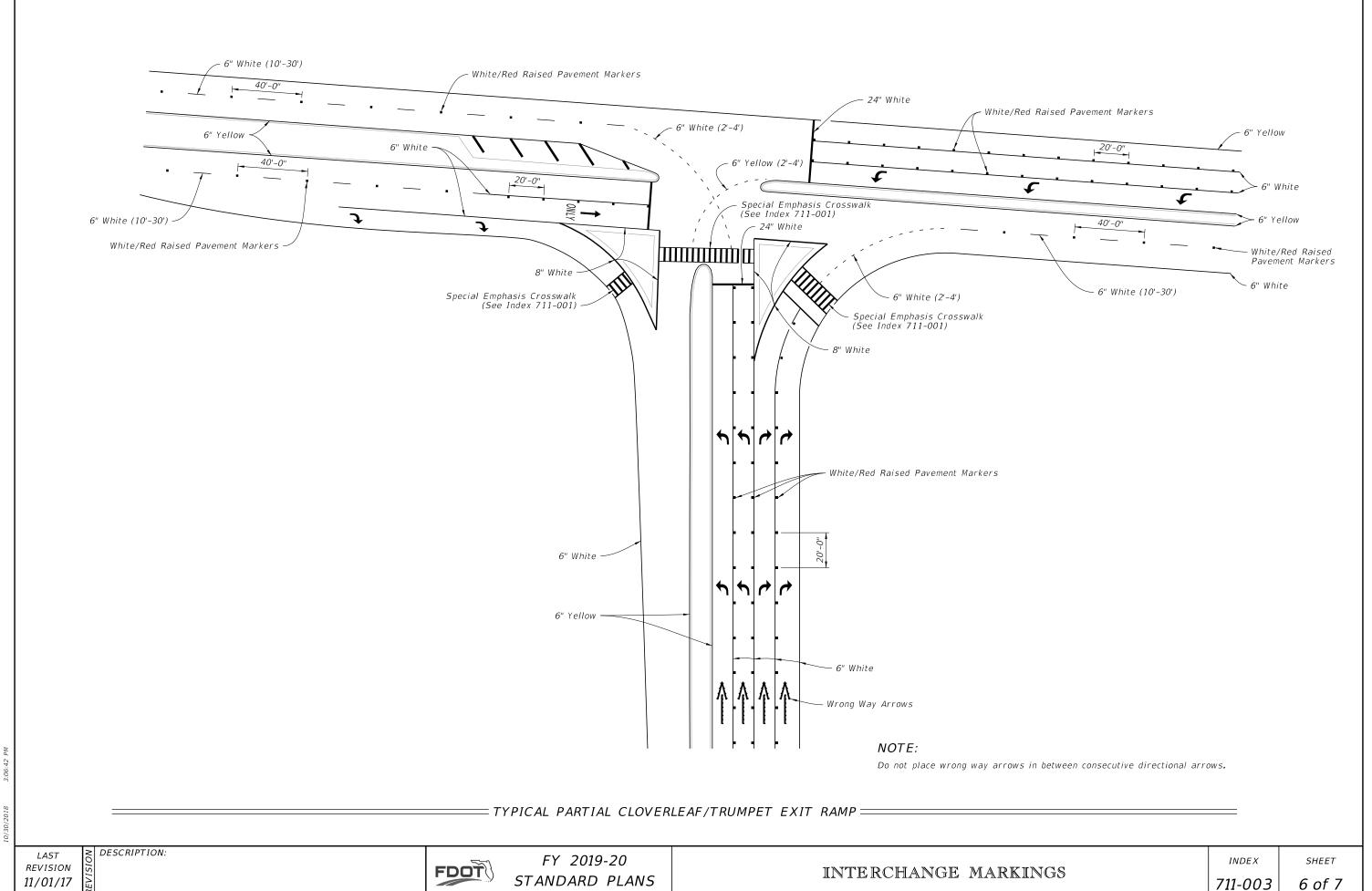


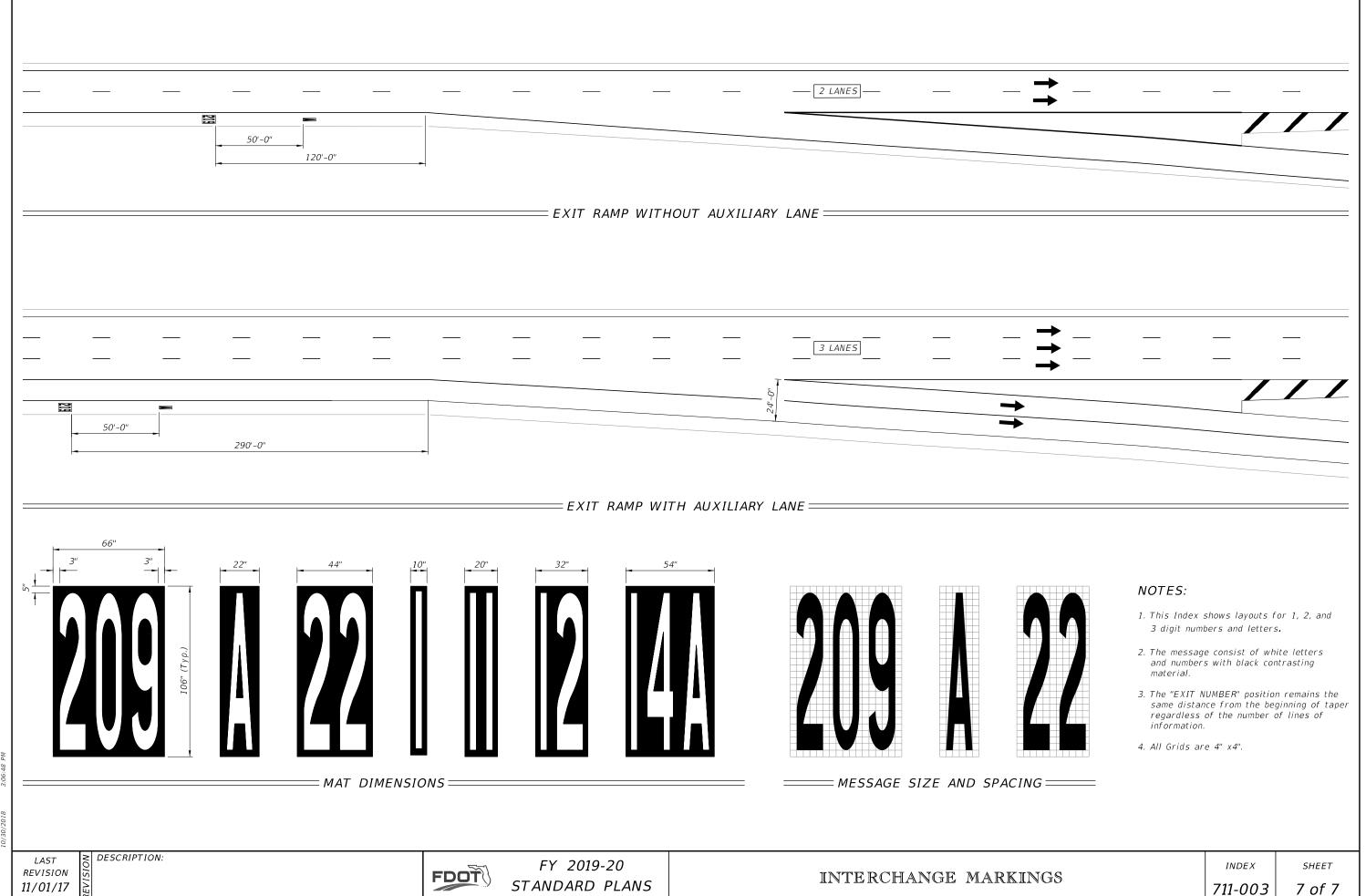






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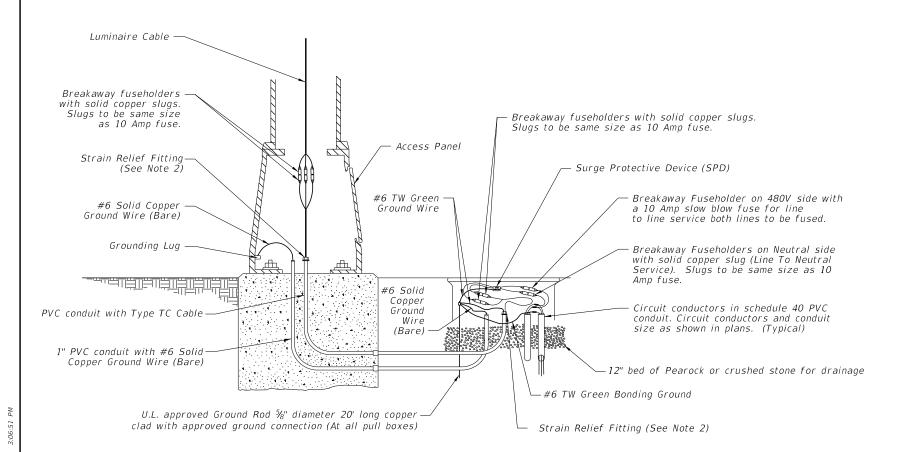


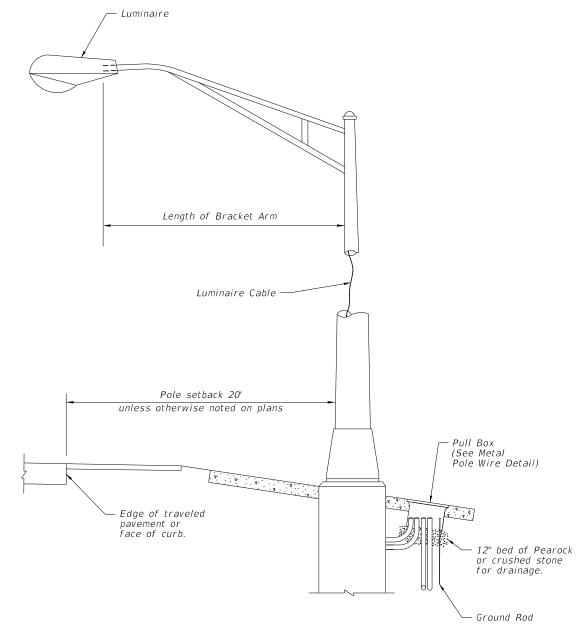


11/01/17

STANDARD PLANS

METAL POLE WIRING DETAIL





### METAL POLE DETAIL

### NOTES:

- 1. Barrier wall or bridge mounted poles: The wiring shall be in accordance with Specification 992.
- 2. Provide cable length to remove fuseholders from transformer base, pole base or pullbox for maintenance. Remove slack from the luminaire cable to provide tension on the fuseholders if the pole breaks away. Pull excess cable into pull box tighten strain relief fittings or cable clamps at both ends of conduit to prevent cable from slipping.

# WIRING DETAILS

**REVISION** 11/01/17

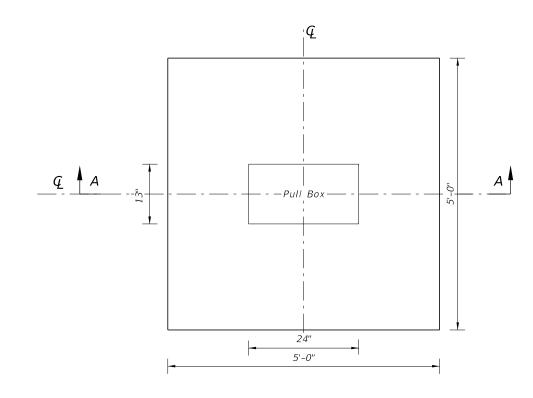
DESCRIPTION:

FDOT

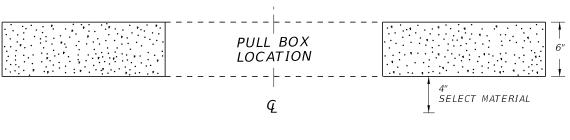
FY 2019-20 STANDARD PLANS CONVENTIONAL LIGHTING 715-001

INDEX SHEET

- 1. Use compacted select material in accordance with Index 120-001.
- 2. Concrete shall be Class NS with a minimum strength at 28 days of f'c=2.5 ksi.
- 3. Outside edge of slab shall be cast against formwork.
- 4. The pull box shown is 13" x 24"; others approved under Specifications 635 may be used.
- Slabs to be placed around all Poles and Pull Boxes in rural locations. In urban areas or where space is limited slab dimensions may be adjusted as shown in the plans.
- 6. Concrete for slabs around pull boxes shall be included in the price of pull box.



SLAB DIMENSIONS



SECTION A-A

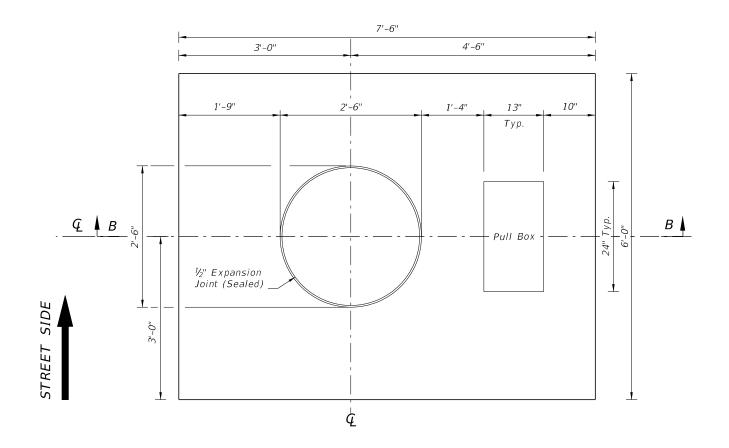
SLAB DETAILS FOR INTERMEDIATE PULLBOX LOCATIONS

LAST REVISION 11/01/17

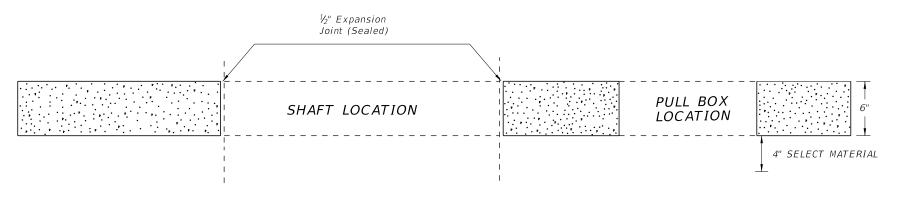
DESCRIPTION:

FDOT

- 1. Use compacted select material in accordance with Index 120-001.
- 2. Concrete shall be Class NS with a minimum strength at 28 days of f'c=2.5 ksi.
- 3. Outside edge of slab shall be cast against formwork.
- 4. The pull box shown is 13" x 24"; others approved under Specification 635 may be used.
- 5. Slabs to be placed around all Poles and Pull Boxes. In urban areas or where space is limited slab dimensions may be adjusted as shown in the plans.
- 6. Concrete for slabs around poles and pull boxes shall be included in the price of pole or pull box.
- 7. The expansion joint shall consist of ½" of closed-cell polyethylene foam expansion material. The top ½" of expansion material shall be removed after pouring the slab and sealed with an APL approved Type A sealant meeting the requirements of Specification 932.



SLAB DIMENSIONS



SECTION B-B

SLAB DETAILS FOR POLE AND PULL BOX LOCATIONS

LAST **REVISION** 11/01/17

DESCRIPTION:

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SHEET 3 of 3

- 2. Shop Drawings: This Index is considered fully detailed, only submit shop drawings for minor modifications not
- 3. Materials:
  - A. Pole, Pole Connection Extrusions and Arm Extrusions: ASTM B221, Alloy 6063-T6 or Alloy 6061-T6
  - B. Bars, Plates, Stiffeners and Backer Ring: ASTM B221, Alloy 6063-T6
  - C. Caps and Covers: ASTM B-26, Alloy 319-F
  - Steel Bearing Plate: ASTM A709 or ASTM A36 Grade 36
  - Aluminum Weld Material: ER 4043
  - Transformer and Frangible Base Materials: ASTM B26 or ASTM B108, Alloy 356-T6

  - G. Bolts, Nuts and Washers: a. Shoe Base Bolts: ASTM F3125, Grade A325, Type 1
    - b. Nuts: ASTM A563 Grade DH Heavy-Hex
  - c. Washer: ASTM F436 Type 1

  - H. Anchor Bolts, Nuts, and Washers: a. Anchor Bolts: ASTM F1554 Grade 55
    - b. Nuts: ASTM A563 Grade A Heavy-Hex
  - c. Plate Washer: ASTM A36
  - I. Stainless Steel Fasteners: ASTM F593 Alloy Group 2, Condition A, CW1 or SH1
  - J. Nut Covers: ASTM B26 (319-F)
  - K. Concrete: Class 1
  - L. Reinforcing Steel: Specification 415
- 4. Fabrication:
  - A. Weld Arm and Pole (Alloy 6063) in the T4 temper using 4043 filler. Age the Arm and Pole artificially to the T6 temper after welding.
  - B. Transverse welds are only allowed at the base.
  - C. Roadway Light Pole Taper: Taper as required to provide a round top 0.D. of 6" and a base 0.D. of 10". Portions of the pole near the base shoe and at the arm connections may be held constant at 10" and 6" respectively to simplify fabrication.
  - D. Median Barrier Mounted Light Pole Taper: Taper as required to provide a 6" O.D. round top with an 11" x 7" O.D. oblong base. Portions of the pole near the base and at the arm connections may be held constant at 11"x 7" oblong and 6" round respectively to simplify fabrication.
  - E. Provide 'J', 'S' or 'C' hook at top of pole for electrical wires.
  - F. Equip poles located on bridges, walls and concrete median barriers/Traffic Railings with a vibration damper.
  - G. Perform all welding in accordance with AWS D1.2.
  - H. Embedded Junction Box (EJB):
    - a. Weld all seams continuously and grind smooth.
    - b. Hot Dip Galvanize after Fábrication.
    - c. Provide a watertight cover with neoprene gasket and secure cover with galvanized screws.
  - I. For Median Barrier Mounted Aluminum Light Poles, the fabricator must demonstrate the ability to produce a crack free pole. The fabricator's Department-approved QC Plan must contain the following information prior to
    - a. Tests demonstrating a pole with a  $V_4$ " wall thickness achieves and ultimate moment capacity of 36 kip*ft in the strong axis and 30 kip*ft in the weak axis.
    - b. Tests demonstrating a pole with a  $\frac{1}{16}$ " wall thickness achieves an ultimate moment capacity of 44 kip*ft in the strong axis and 37 kip*ft in the weak axis.
    - c. Test results showing the pole does not buckle at the shape transition area under the ultimate moment capacity loads.
  - d. Complete details and calculations for the reinforced 4"x 6" (Min.) handhole located 1'-6" above the base plate. J. Identification Tag: (Submit details for approval.)

    - a. 2" x 4" (Max.) aluminum identification tag. b. Locate on the inside of the transformer base and visible from the door opening.
    - c. Secure to transformer base with 1/8" diameter stainless steel rivets or screws.
    - d. Include the following information on the ID Tag:
      - 1. Financial Project ID
      - 2. Pole Height
      - 3. Manufacturer's Name

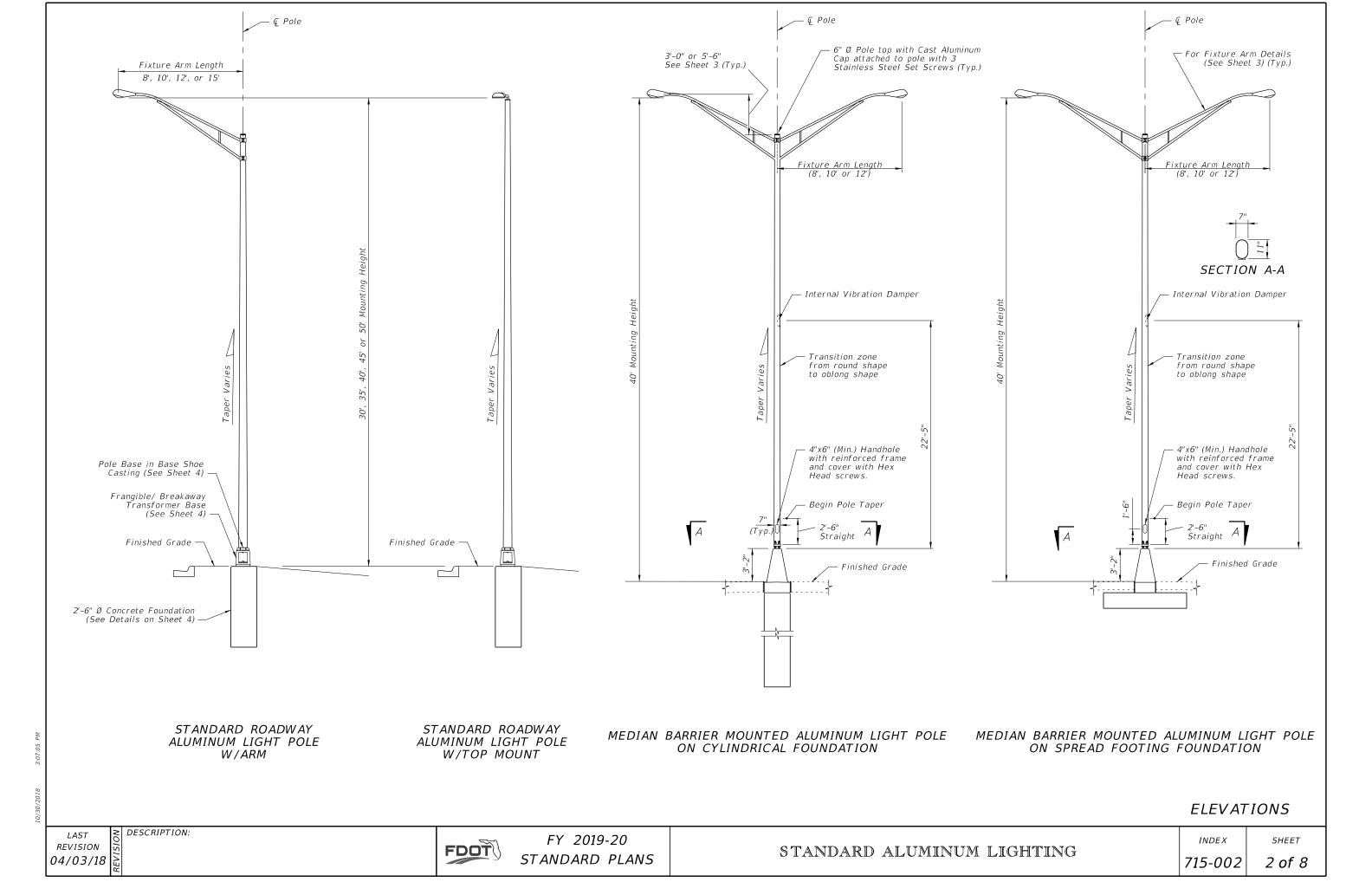
- 5. Coatings/Finish:
  - A. Pole and Arm Finish: 50 grit satin rubbed.
  - B. Galvanize Steel Bolts, Screws, Nuts and Washers: ASTM F2329
  - C. Hot Dip Galvanize EJB and other steel items including poles and plate washers: ASTM A123
- 6. Construction:
  - A. Foundation: Specification 455, except payment for the foundation is included in the cost of the pole.
  - B. Frangible Base, Base Shoe, and Clamb:
    - a. Certify that the Clamp, Frangible Transformer Base, and Base Shoe Design are capable of providing the required capacity.
    - b. Certify the Base conforms to the current FHWA required AASHTO Frangibility Requirements, tested under NCHRP Report 350 Guidelines (e.g. Akron Foundry TB1-17).
    - c. Do not erect pole without Luminaire attached.
- 7. Embedded Junction Box (EJB): Install EJBs per Note 4 and in accordance with Specification 635, as shown on the following Sheets.
- 8. Wind Speed by County:

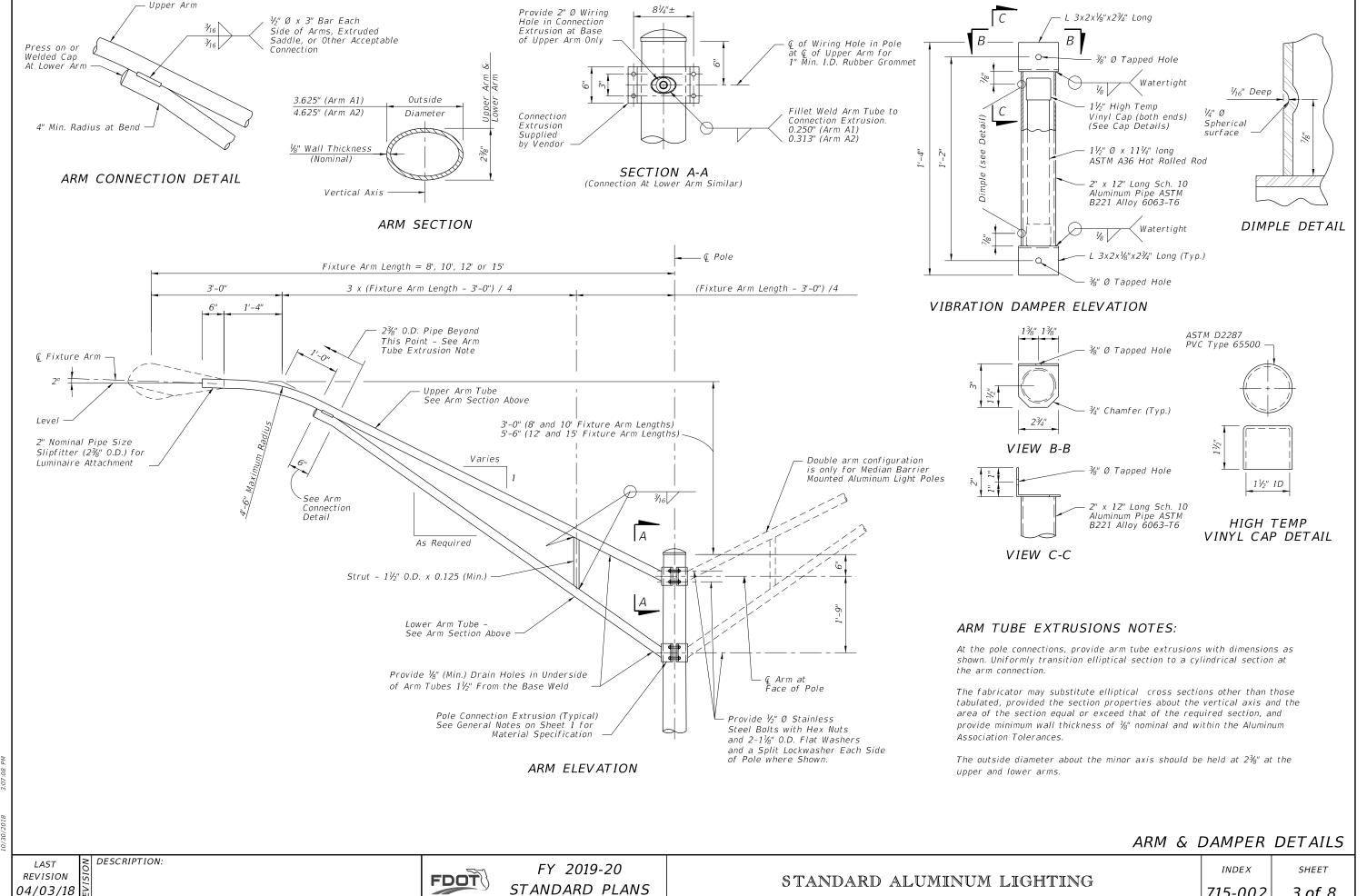
Alachua, Baker, Bradford, Calhoun, Clay, Columbia, Dixie, Duval, Gadsden, Gilchrist, Hamilton, Jackson, Jefferson, Lafayette, Leon, Liberty, Nassau, Madison, Putnam, Suwannee, Taylor, Union and Wakulla Counties.

Bay, Citrus, De Soto, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Holmes, Lake, Levy, Manatee, Marion, Okaloosa, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Santa Rosa, Seminole, St. Johns, Sumter, Volusia, Walton and Washington Counties.

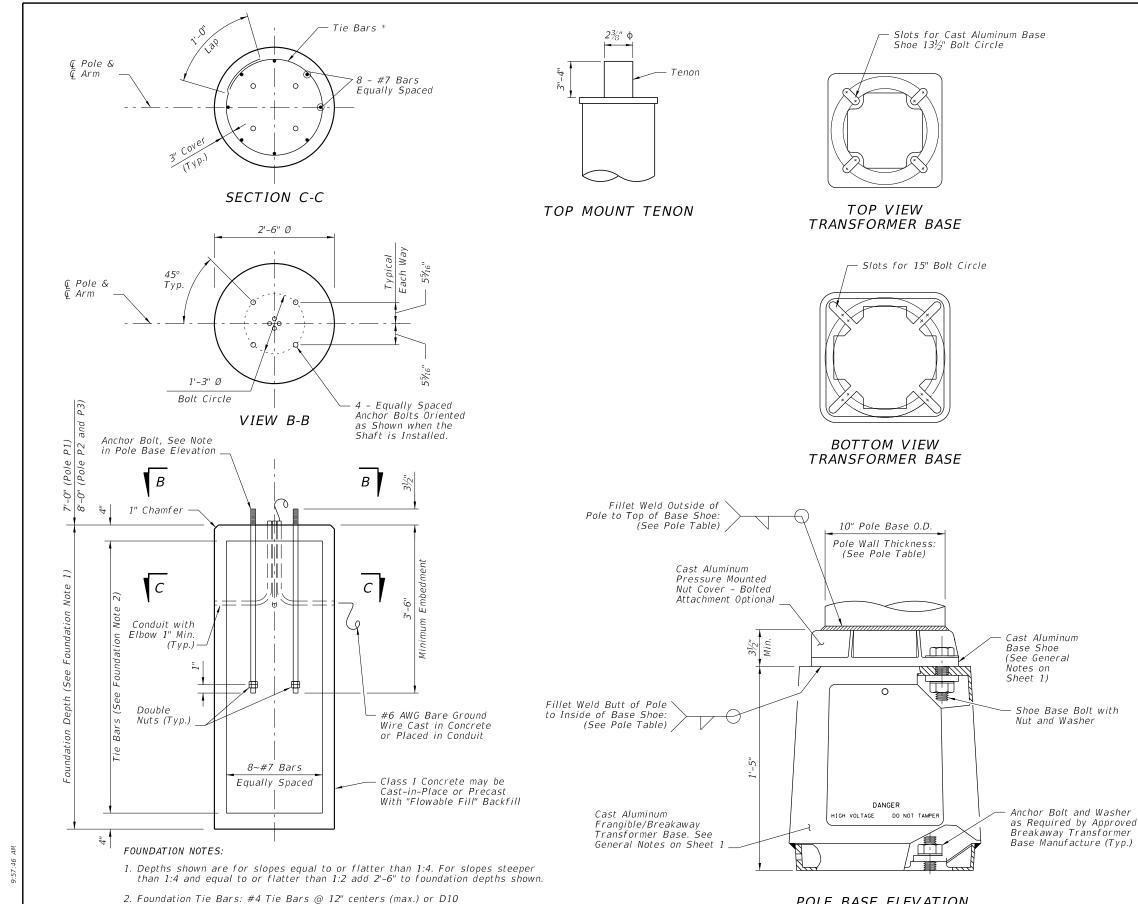
Brevard, Broward, Charlotte, Collier, Escambia, Indian River, Lee, Martin, Miami-Dade, Monroe, Palm Beach, Sarasota and St. Lucie Counties.

DESCRIPTION:





STANDARD PLANS



# ARM-POLE TABLE

### FOR STANDARD ALUMINUM LIGHT POLES WITH ARM

Assembly	Wind Speed and Arm Lengths (ft)								
Height	120 mph			140 mph			160 mph		
(ft)	8,	10,	12,	15	8, 10, 1	2	15	8, 10	12, 15
30						П		A1-P1	A2-P1
35		Λ1	-P1	D 1	A1-P1	A2-P1	AI-FI	A2-P1	
40		AI	-P I					A1-P2	A2-P2
45		Λ1	-P2		A1-P2		A2-P2	A1-P2	A2-P2
50		AI.	-22			AZ-PZ	A1-P3	A2-P3	

### ARM POLE NOTES:

- 1. See ARM SECTION detail on Sheet 3 for all A1 and A2 Values.
- 2. See Pole Table for all P1, P2, and P3 values.
- 3. For Median Barrier Mounted Pole, Use Arm A1.

POLE TABLE								
Pole	Pole Wall Thickness	Top of Base Shoe Weld	Inside of Base Shoe Weld					
P1	0.156	³ / ₁₆ "	5⁄ ₃₂ "					
P2	0.250	1/4"	1/4"					
Р3	0.313	5∕ ₁₆ "	5/ ₁₆ "					

### POLE NOTES:

- 1. Pole wall thicknesses shown are nominal and must be within the Aluminum Association tolerances.
- 2. Thicker walls are permitted and tapered walls may be used in accordance with the minimum Aluminum Association thicknesses.

TOP MOUNT POLE TABLE  FOR STANDARD ALUMINUM LIGHT POLES  WITH TOP MOUNT							
Assembly Wind Speed and Arm Lengths (ft)							
Height (ft)	120 mph	160 mph					
30			Pole P1				
35	Pole P1	Pole P1					
40							
45	Pole P2	Pole P2	Pole P2				
50	ruie P2	Fole P2					

# POLE BASE ELEVATION

# FOUNDATION

(or W10) spiral @ 6" pitch, 3 flat turns top and 1 flat turn bottom.

# POLE AND BASE DETAILS FOR ROADWAY ALUMINUM LIGHT POLE

**REVISION** 04/03/18

DESCRIPTION:

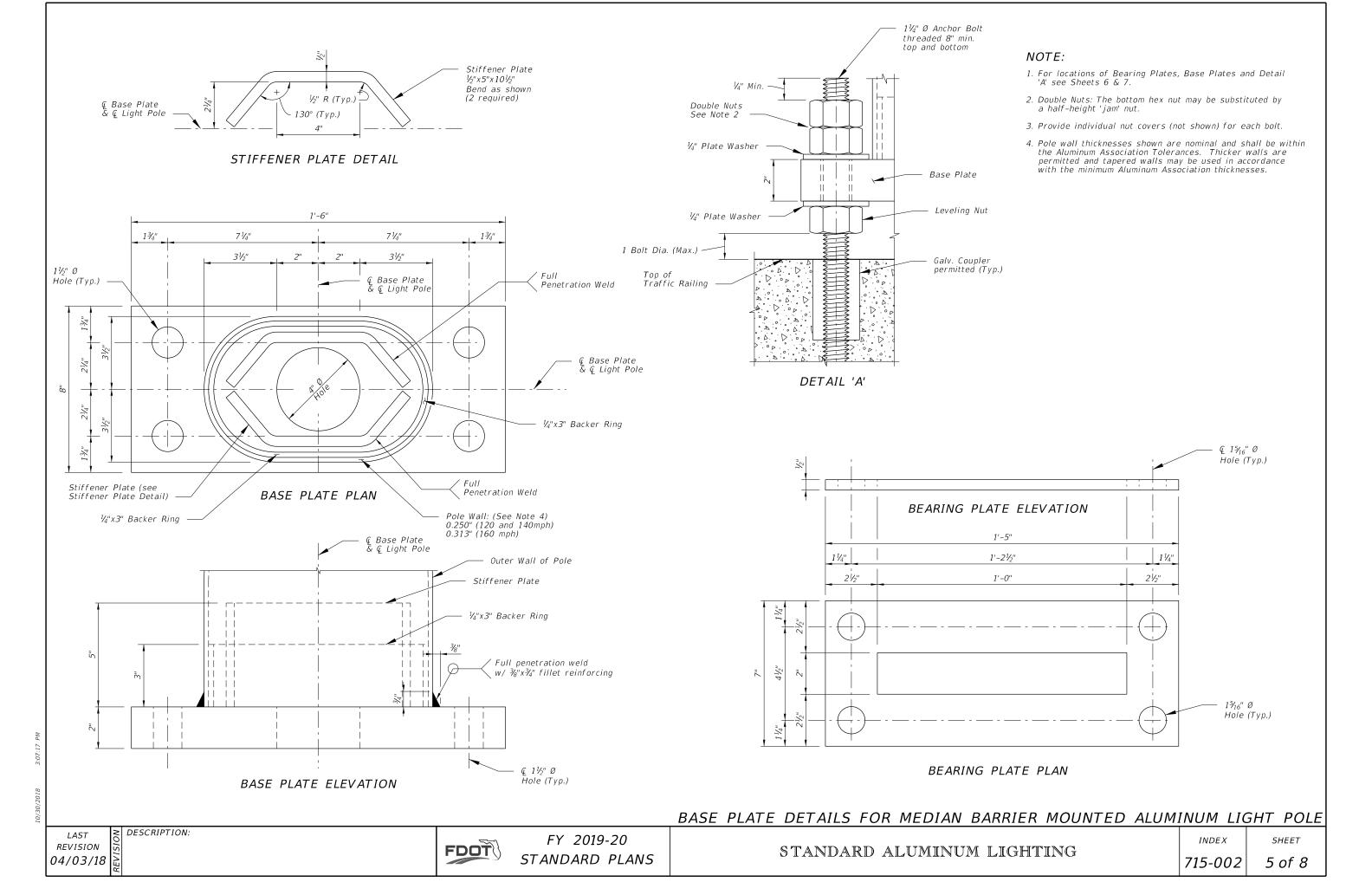
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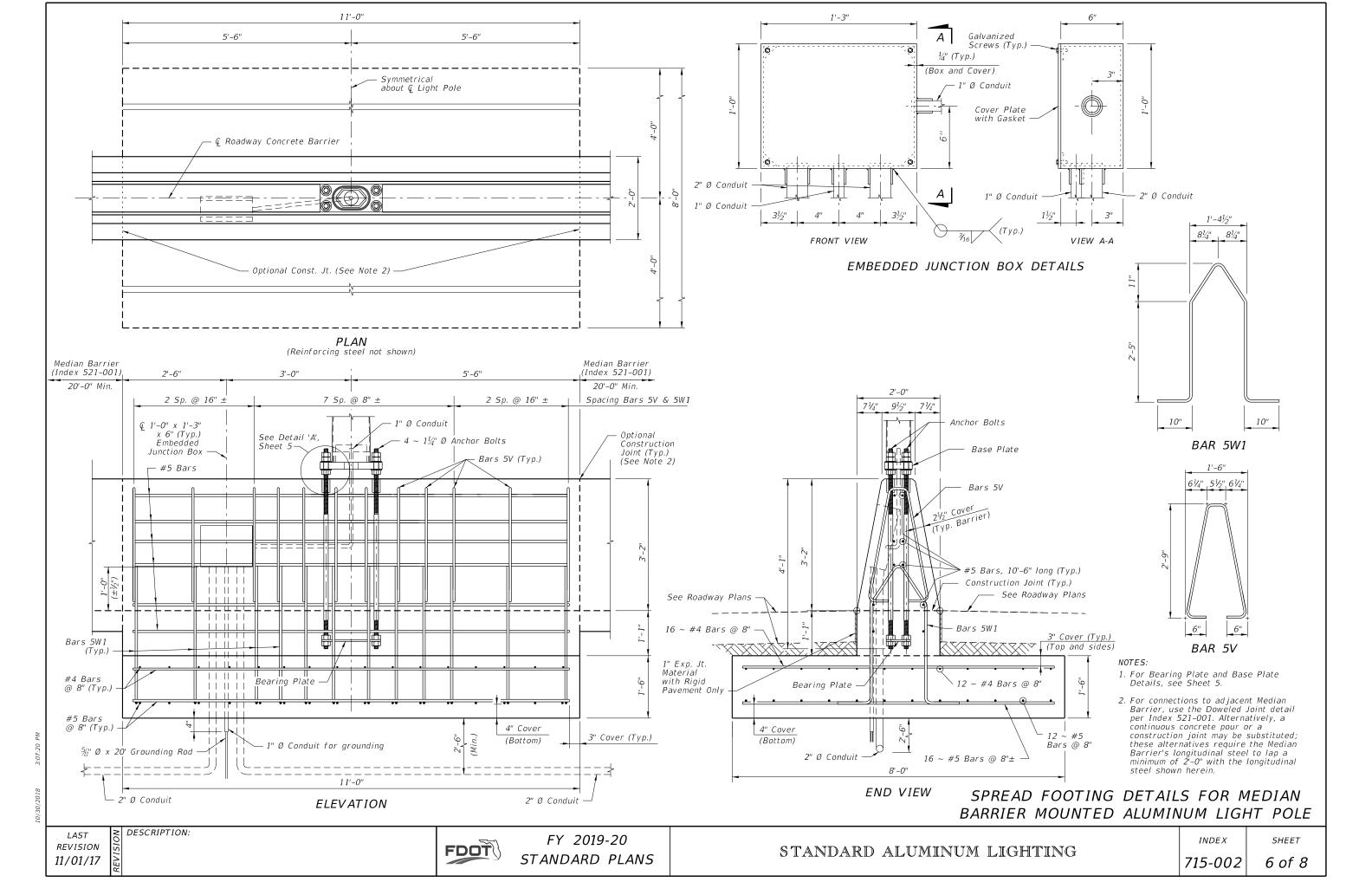
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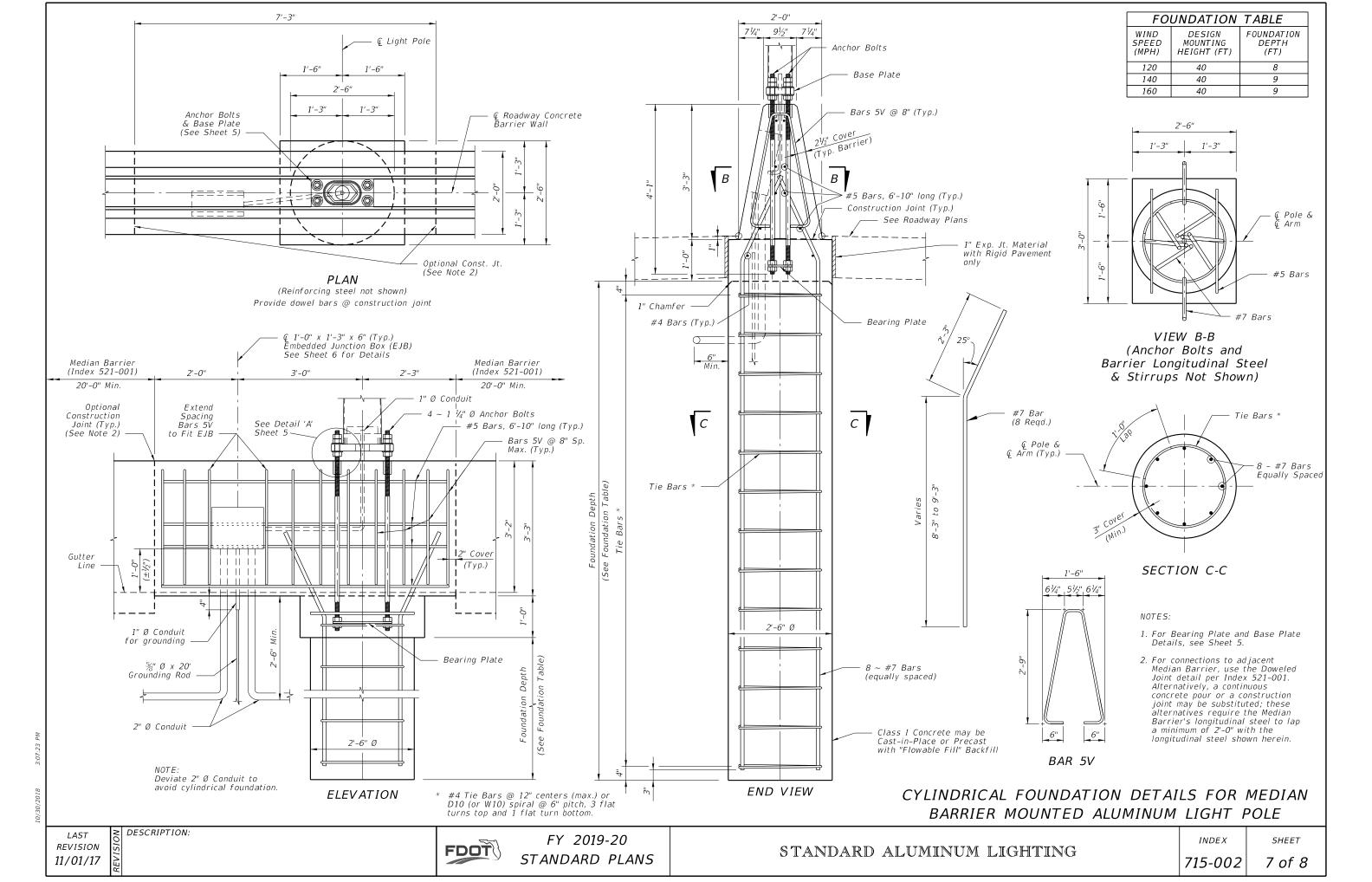
STANDARD ALUMINUM LIGHTING

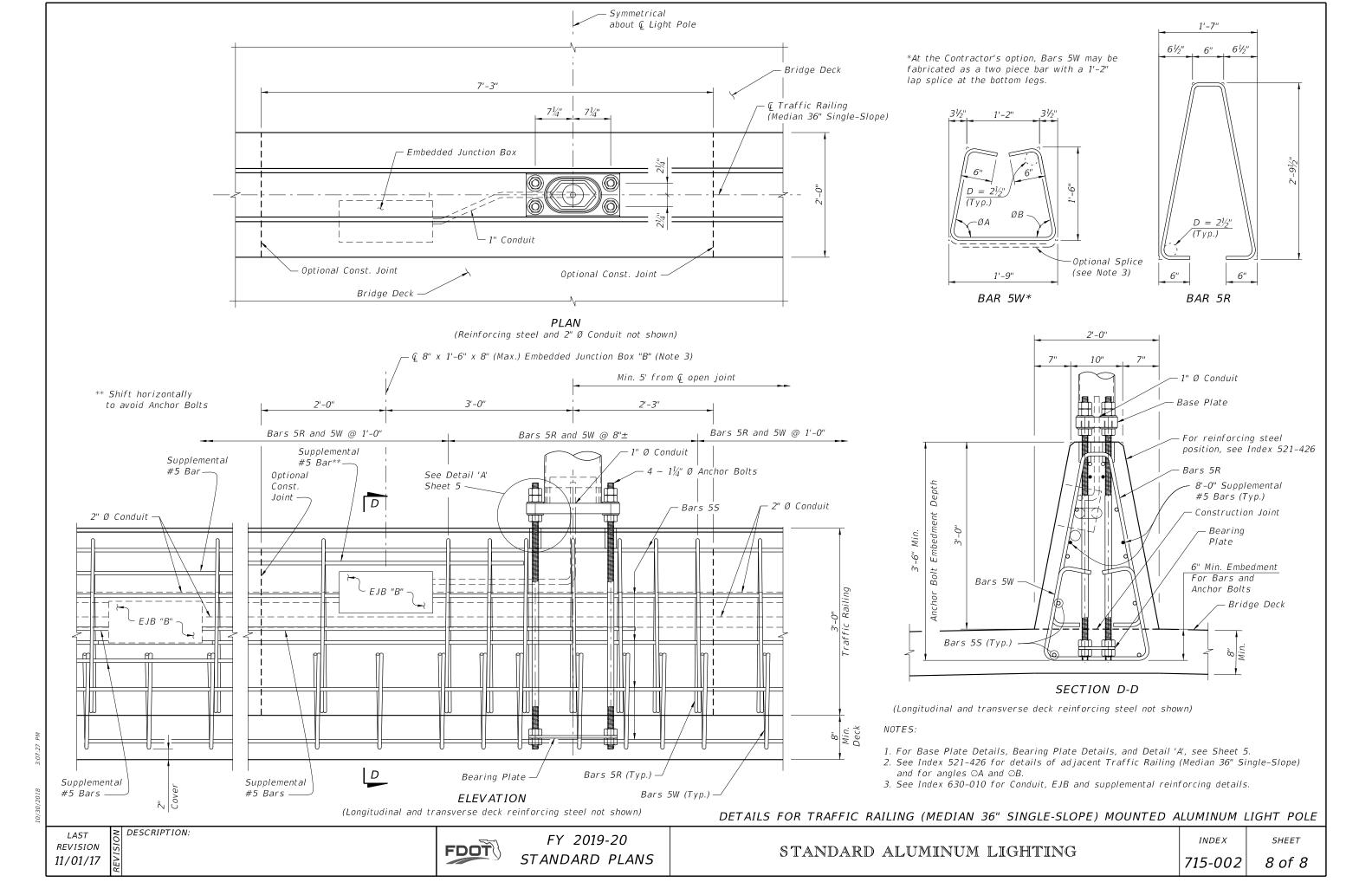
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B. Eight (8) cylindrical luminaires with a maximum effective projected are of 1.5 sf and 77 lbs each.

- 2. Shop Drawings: This Index is considered fully detailed, only submit shop drawings for minor modifications not detailed in the Plans.
- 3. High Mast Structure Materials:
  - A. Poles and Backing Rings:
    - a. Less than  $\frac{3}{16}$ ": ASTM A1011 Grade 50, 55, 60 or 65
    - b. Greater than or equal to  $\frac{3}{16}$ ": ASTM A572 Grade 50, 55, 60 or 65

  - c. ASTM A595 Grade A (55 ksi yield) or Grade B (60 ksi yield) B. Steel Plates: ASTM A709 or ASTM A36 C. Pole Caps: ASTM A1011 Grade 50, 55, 60, or 65 or ASTM B209
  - D. Weld Metal: E70XX
  - E. Stainless Steel Screws: AISI 316
  - F. Anchor Bolts, Nuts and Washers:
    - a. Anchor Bolts: ASTM F1554 Grade 55
    - b. Nuts: ASTM A563 Grade A Heavy-Hex (5 per anchor bolt)
    - c. Plate Washer: ASTM A36 (2 per anchor bolt)
  - G. Nut Covers: ASTM B26 (319-F)
  - H. Concrete: Class IV (Drilled Shaft)
  - I. Reinforcing Steel: Specification 415
- 4. Fabrication:
  - A. Welding:
  - a. Specification Section 460-6.4 and
  - b. AASHTO LRFD Specification for Structural Supports for Highway Signs, Luminaires, and Traffic
  - Signals Section 14.4.4

  - a. Round or 16-sided (Min.)
  - b. Taper pole diameter at 0.14 inches per foot
  - c. Pole shaft may be up to three sections (using telescopic field splices)
  - d. Circumferentially welded pole shafts and laminated pole shafts are not permitted
  - e. Fabricate Pole longitudinal seam welds (2 maximum) with 60 percent minimum penetration or fusion welds except as follows:
  - i. Use a full-penetration groove weld within 6 inches of the circumferential tube-to-plate connection and ii. Use full-penetration groove welds on the female end section of telescopic (i.e., slip type) field
  - splices for a minimum length of 42 inches. C. Identification Tag: (Submit details for approval)

  - a. 2"x 4" (Max.) aluminum tag
  - b. Locate on the inside of the pole and visible from the handhole c. Secure with 1/8" diameter stainless steel rivets or screws.

  - d. Include the following information on the ID Tag: 1. Financial Project ID

    - 2. Pole Type
    - 3. Pole Height
    - 4. Manufacturers' Name 5. Yield Strength (Fy of Steel)
    - 6. Base Wall Thickness
  - D. Except for Anchor Bolts, bolt hole diameters are bolt diameter plus 1/16" and anchor bolts holes are
  - bolt diameter plus ½" (Max) prior to galvanizing. E. Hot Dip Galvanize after fabrication
- - A. Galvanize Anchor Bolts, Nuts and Washers: ASTM F2329
  - B. Hot Dip Galvanize all other steel items including plate washers: ASTM A123
- - A. Foundation: Specification 455 Drilled Shaft, except that payment is included in the cost of the Structure.
  - B. After Installation: Place wire screen between top of foundation and bottom of baseplate in accordance with Specification 649-6.
- 7. Wind Speed by County:

Alachua, Baker, Bradford, Calhoun, Clay, Columbia, Dixie, Duval, Gadsden, Gilchrist, Hamilton, Jackson, Jefferson, Lafayette, Leon, Liberty, Nassau, Madison, Putnam, Suwannee, Taylor, Union and Wakulla Counties.

Bay, Citrus, De Soto, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Hólmes, Lake, Levy, Manatee, Marion, Okaloosa, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Santa Rosa, Seminole, St. Johns, Sumter, Volusia, Walton and Washington Counties.

Brevard, Broward, Charlotte, Collier, Escambia, Indian River, Lee, Martin, Miami-Dade, Monroe, Palm Beach, Sarasota and St. Lucie Counties.

STANDARD POLE DESIGN NOTES

LAST **REVISION** 11/01/18

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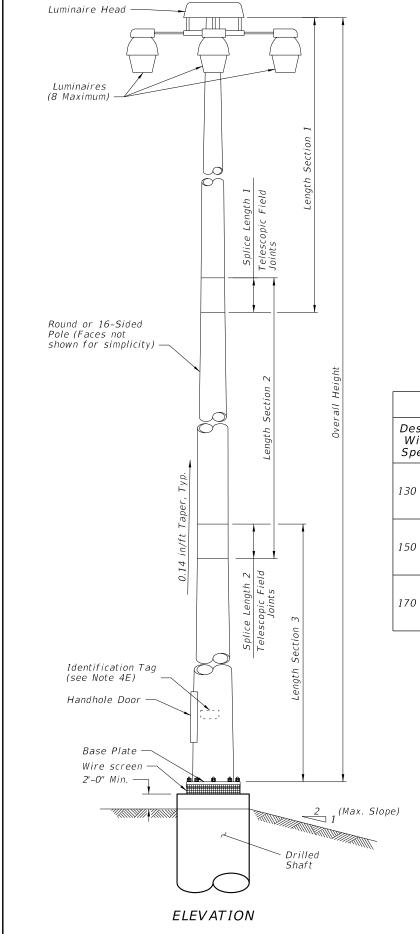
FY 2019-20 STANDARD PLANS

HIGH MAST LIGHTING

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POLE DESIGN TABLE*													
- ·		SECTION 1 (TOP)					SECTION 2				SECTION 3		
Design Wind Speed	Pole Overall Height (ft)	Length	Wall Thickness (in.)	Minimum Splice Length 1	Base Dia. (in.)	Length	Wall Thickness (in.)	Minimum Splice Length 2	Base Dia. (in.)	Length	Wall Thickness (in.)	Base Dia. (in.)	
	80	41'-0"	0.250	2'-0"	11	42'-0"	0.250		16	_	_	_	
130 mph	100	23'-0"	0.179	2'-0"	10	41'-0"	0.250	2'-6"	15	43'-0"	0.250	20	
	120	41'-0"	0.250	2'-0"	12	43'-0"	0.250	2'-9"	17	43'-0"	0.313	22	
	80	41'-0"	0.250	2'-0"	11	42'-0"	0.313		16	_	_		
150 mph	100	23'-0"	0.179	2'-0"	10	41'-0"	0.250	2'-6"	15	43'-0"	0.313	20	
	120	41'-0"	0.250	2'-6"	16	43'-0"	0.250	3'-0"	21	44'-0"	0.375	26	
	80	40'-0"	0.250	2'-3"	13	43'-0"	0.313		18				
170 mph	100	23'-0"	0.250	2'-0"	11	42'-0"	0.313	2'-6"	16	44'-0"	0.375	21	
	120	41'-0"	0.250	3'-0"	18	44'-0"	0.313	3'-6"	23	45'-0"	0.375	28	

^{*} Diameter Measured Flat to Flat

	BASE PLATE AND BOLTS DESIGN TABLE							
Design Wind Speed	Pole Overall Height (ft)	Base Plate Diameter (in.)	Base Plate Thickness (in.)	Bolt Circle (in.)	No. Bolts	Bolt Diameter (in.)	Bolt Embedment (in.)	
	80	30.0	3.000	23.0	8	1.75	38	
130 mph	100	34.0	3.000	27.0	8	1.75	42	
	120	38.0	3.875	30.0	8	2.00	48	
	80	30.0	3.000	23.0	8	1.75	43	
150 mph	100	36.0	3.875	28.0	8	2.00	47	
	120	44.0	3.875	35.0	8	2.25	52	
	80	32.0	3.000	25.0	8	1.75	47	
170 mph	100	37.0	3.000	29.0	8	2.00	54	
	120	46.0	3.875	37.0	10	2.25	58	

	SHAFT DESIGN TABLE								
Design Wind Speed	Pole Overall Height (ft)	Shaft Diameter	Shaft Length	Longitudinal Reinforcement					
	80	4'-0"	13'-0"	14-#11					
130 mph	100	4'-6"	14'-0"	16-#11					
	120	4'-6"	16'-0"	16-#11					
	80	4'-0"	14'-0"	14-#11					
150 mph	100	4'-6"	16'-0"	16-#11					
	120	5'-0"	18'-0"	18-#11					
	80	4'-6"	15'-0"	16-#11					
170 mph	100	4'-6"	17'-0"	16-#11					
	120	5'-0"	20'-0"	18-#11					

### NOTE.

Shaft Design Table Shaft Length is based on level ground (flatter than 1:5). Increase the shaft depth in accordance with the Additional Shaft Depth Due to Ground Slope table for foundations with slopes 1:5 and steeper. Use the higher value for slope or diameter values that fall between those shown on the table.

,	ADDITIONAL SHAFT DEPTH DUE TO GROUND SLOPE							
Ground Slope	4'-0" Shaft Diameter	5'-0" Shaft Diameter						
1:5	3'-0"	4'-0"						
1:4	4'-0"	5'-0"						
1:3	5'-0"	6'-0"						
1:2	7'-0"	9'-0"						

POLE DESIGN TABLES

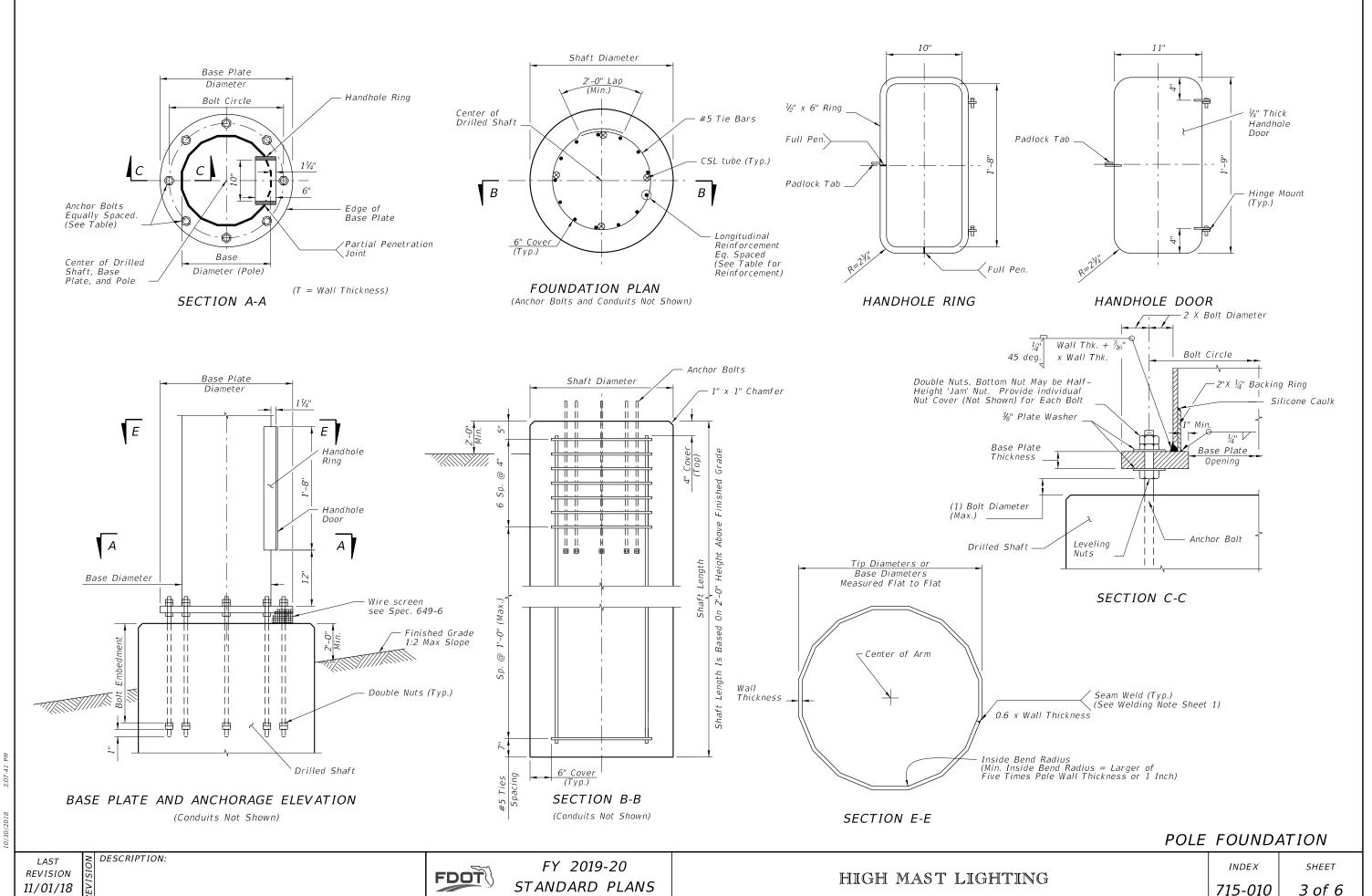
LAST REVISION 11/01/18

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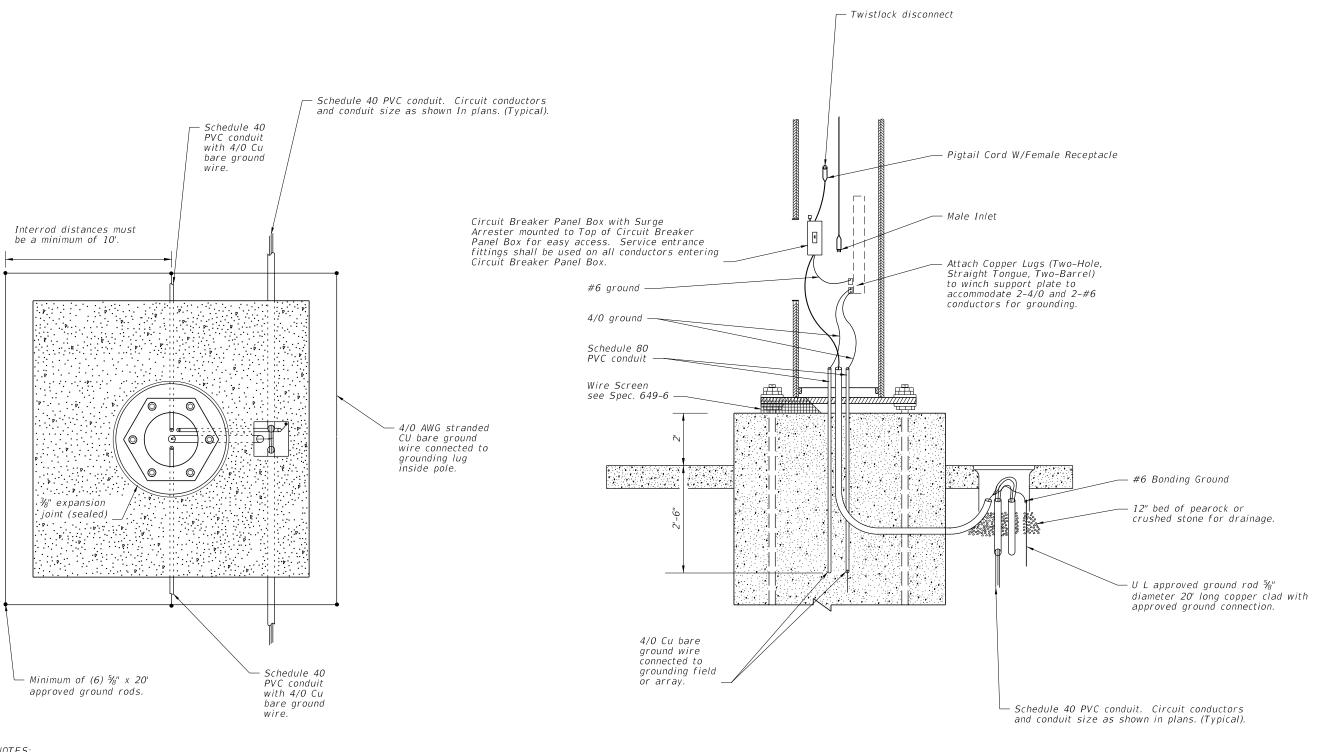
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STANDARD PLANS



DESCRIPTION:

- 1. At all pull boxes and pole bases, ends of conduit shall be sealed in accordance with Section 630 of the Standard Specifications For Road And Bridge Construction.
- 2. Slabs to be placed around all Poles and Pull Boxes.
- 3. For Pull Boxes between Poles refer to Index 715-001.

WIRING DETAILS

LAST **REVISION** 11/01/17

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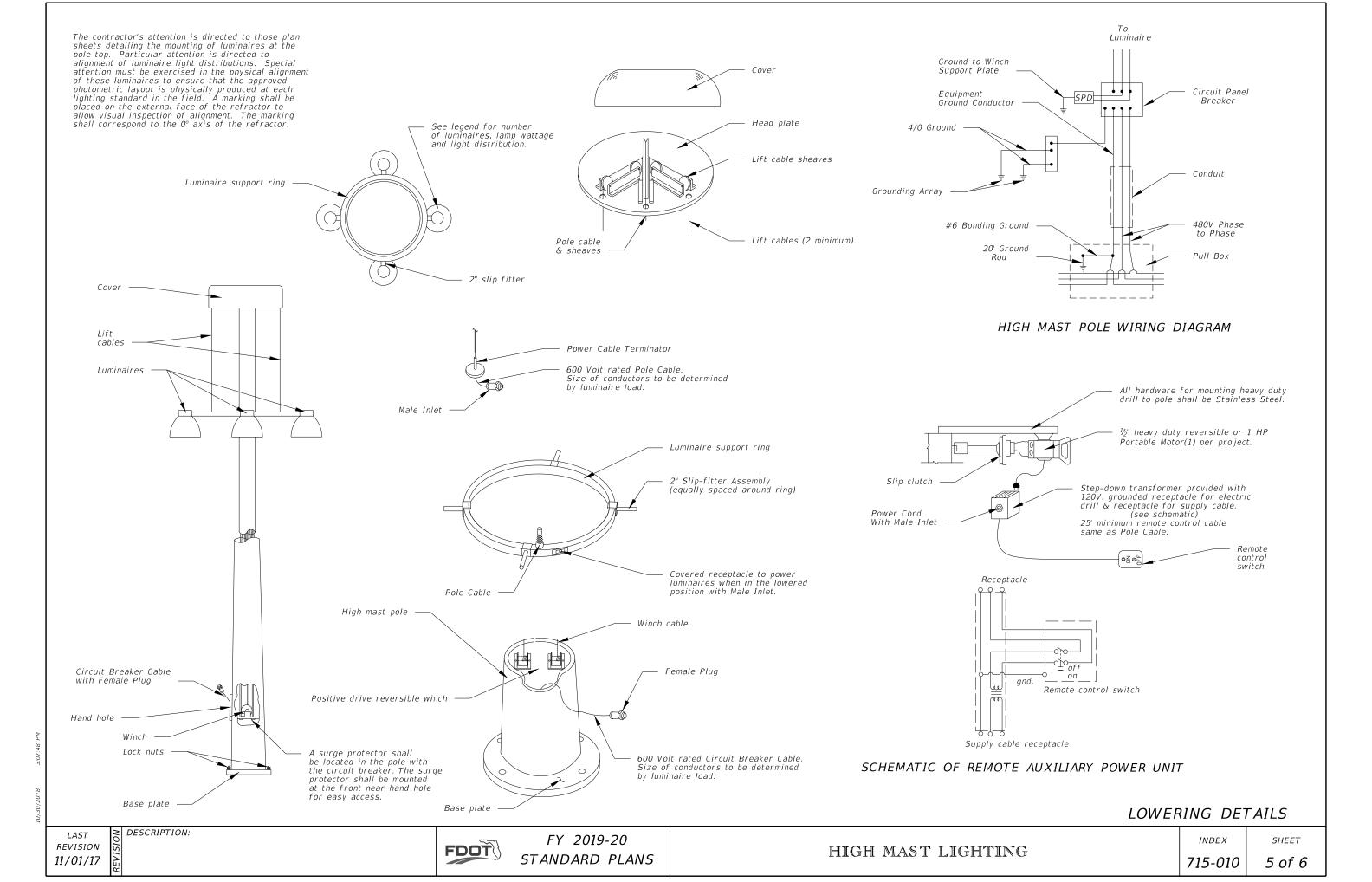
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HIGH MAST LIGHTING

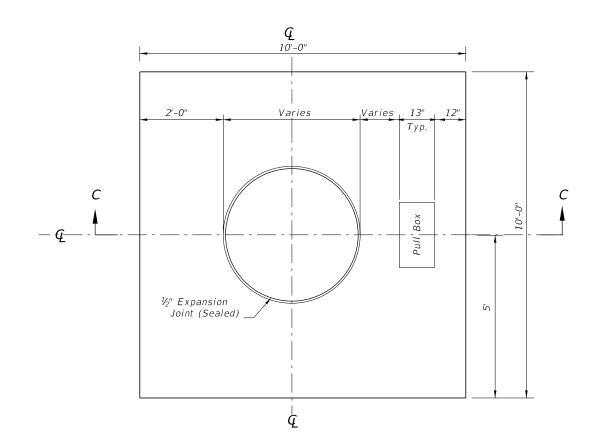
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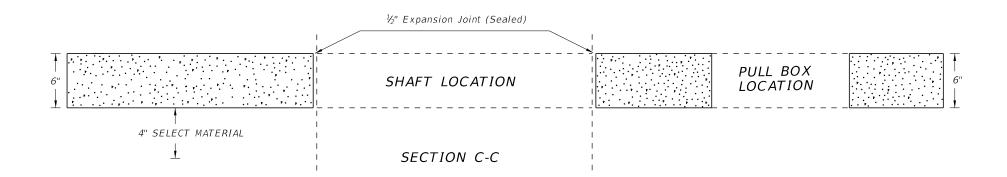
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- 1. Use compacted select material in accordance with Index 120-001.
- 2. Concrete shall be Class NS with a minimum strength at 28 days of f'c=2.5 ksi.
- 3. Outside edge of slab shall be cast against formwork.
- 4. The pull box shown is 13" x 24"; others approved under Specification 635 may be used.
- 5. Slabs to be placed around all Poles and Pull Boxes. In urban areas or where space is limited slab dimensions may be adjusted as shown in the plans.
- 6. Concrete for slabs around poles and pull boxes shall be included in the price of pole or pull box.
- 7. The expansion joint shall consist of  $\frac{1}{2}$ " of closed-cell polyethylene foam expansion material. The top  $\frac{1}{2}$ " of expansion material shall be removed after pouring the slab and sealed with an APL approved Type A sealant meeting the requirements of Specification 932.



SLAB DIMENSIONS



SLAB DETAILS

**REVISION** 11/01/17

DESCRIPTION:

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CROSSING SURFACES						
Definition						
Concrete						
Rubber						
Rubber/Asphalt						
Timber/Asphalt						

STOP ZONE FOR	RUBBER CROSSING
Design Speed (mph)	Zone Length (Distance From Stop)
45 Or Less	250'
50 - 55	350'
60 - 65	500'
70	600'

## Notes:

- 1. Type R Crossings are NOT to be used for multiple track crossings within zones for an existing or scheduled future vehicular stop. Zone lengths are charted above.
- 2. Single track Type R Crossings within the zones on the chart may be used unless engineering or safety considerations dictate otherwise.

## GENERAL NOTES

- 1. The Railroad Company will furnish and install all track bed (ballast), crossties, rails, crossing surface panels and accessory components. All pavement material, including that through the crossing, will be furnished and installed by the Department or its Contractor, unless negotiated otherwise.
- 2. When a railroad grade crossing is located within the limits of a highway construction project, a transition pavement will be maintained at the approaches of the crossing to reduce vehicular impacts to the crossing. The transition pavement will be maintained as appropriate to protect the crossing from low clearance vehicles and vehicular impacts until the construction project is completed and the final highway surface is constructed.
- 3. The Central Rail Office will maintain a list of currently used Railroad Crossing Products and will periodically distribute the current list to the District Offices as the list is updated.
- 4. The Railroad Company shall submit engineering drawings for the proposed crossing surface type to the Construction Project Engineer and/or the District Rail Office for concurrence along with the List of Railroad Crossing Products. The approved engineering drawings of the crossing surface type shall be made a part of the installation agreement.
- 5. Sidewalks shall be constructed through the crossing between approach sidewalks of the crossing. Sidewalks shall be constructed with appropriate material to allow unobstructed travel through the crossing in accordance with ADA requirements.
- 6. Install pavement in accordance with the Specifications.
- 7. The Department will participate in crossing work, that requires adjustments to rail outside of the crossing, no more than 50 feet from the edge of the travel way.

