

LAST REVISION 11/01/17



FY 2018-19 STANDARD PLANS

MISCELLANEOUS EARTHWORK D

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11/01/17

STANDARD PLANS





STANDARD PLANS

SUPERELEVATION - HIGH SPEED ROADWAYS



0.02

SUPERELEVATION RATES (e) FOR URBAN HIGHWAYS AND HIGH SPEED URBAN STREETS

e _{max.}= 0.05



11:09:38 AM

 $e_{max} = 0.05$

SUPERELEVATION FOR URBAN HIGHWAYS AND HIGH SPEED URBAN STREETS

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FY 2018-19 STANDARD PLANS

SUPERELEVATION - LOW SPEED H

GENERAL NOTES

1. Maximum rate of superelevation for urban highways and high speed urban streets shall be 0.05.

2. Superelevation shall be obtained by rotating the plane successively about the break points of the section until the plane has attained a slope equal to that required by the chart. Should the rotation traverse the entire section and further superelevation be required, the remaining rotation of the plane shall be about the low edge of the inside travel lane. Crown is to be removed in the auxiliary lane to the outside of the curve only when the adjoining travel lanes require positive superelevation.

3. When positive superelevation is required, the slope of the gutter on the high side shall be a continuation of the slope of the superelevated pavement.

4. In construction, short vertical curves shall be placed at all angular profile breaks within the limits of the superelevation transition.

5. The variable superelevation transition length "L" shall have a minimum value of 50 feet for design speeds under 40 MPH and 75 feet for design speeds of 40 MPH or greater.

6. Roadway sections having lane arrangements different from those shown, but composed of a series of planes, shall be superelevated in a similar manner.

7. For superelevation of lower speed urban streets, see the FDOT 'Manual Of Uniform Minimum Standards For Design, Construction And Maintenance For Streets And Highways'. For superelevation of curves on rural highways, urban freeways and high speed urban highways, see Index 000-510.

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		_
Line 1	Line 3	Line 2
		Line 4
Point A Point B		
 e 1 - Max. Superelevation Rate (e 2 - Slope Of Parabola At Inside e 3 - Positive Superelevation Rate (Max. Slope Of Parabola.) e 4 - Adverse Superelevation. 	(0.05) de Edge Of Pavl ate Less Than	
evation rates obtained from the of applicable to a parabolic crown perelevation is established by ro he parabolic crown until the deso n sketch). The normal parabolic the limits of the plane thus form	chart or table c section. When t otating a tanger ired slope is at crown will be n ned.	n Sheet 1 his section is nt about the tained (points naintained
PARABOLIC SECTION		
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STANDARD PLANS

SUPERELEVATION - LOW SPEED H

LINE	DESCRIPTION
А	Inside Travel Lane
В	Inside Lane Line
С	Inside Median Edge Pavement
D	€ Construction
Е	Outside Median Edge Pavement
F	Outside Lane Line
G	Outside Travel Lane
Inside	And Outside Are Relative
To Cur	ve Center

*d (Slope	e Ratio)
30 MPH	1: 100
40 MPH	1: 125
45-50 MPH $ m extsf{}$	1: 150

 \triangle 1: 125 May Be Used For 45 MPH Under Restricted Conditions.

r Theoretical Grade Line

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For Corner Clearnace (C) Requirements see General Note 3.

For Additional Information Refer To FDOT Rules Chapters 14-96 And 14-97. SKETCH ILLUSTRATING DEFINITIONS

	CURBED ROADWAYS		FLUSH SHOULDER ROADWAYS			
ELEMENT DESCRIPTION	1-20 Trips/Day or	21-600 Trips/Day or 6-60 Trips/Hour	601-4000 Trips/Day 🖬 or 61-400 Trips/Hour	1-20 Trips/Day or	21-600 Trips/Day or 6-60 Trips/Hour	601-4000 Trips/Day or 61-400 Trips/Hour
	1-5 TTIPS/Hour	2-Way 🗆	2-Way 🗆	1-5 Trips/Hour	2-Way 🗆	2-Way 🗆
CONNECTION WIDTH W	12' Min. 24' Max.	24' Min. 36' Max. ☆	24' Min. 36' Max. ☆	12' Min. 24' Max.	24' Min. 36' Max. ☆	24' Min. 36' Max. ☆
FLARE (Drop Curb) F	10' Min.	10' Min.	N/A	N/A	N/A	N/A
RETURNS (Radius) R & U	N/A	Δ	25' Min. 50' Std. 75' Max.	15' Min. 25' Std. 50' Max.	25' Min. 50' Std. 75' Max.	25' Min. 50' Std. (Or 3-Centered Curves)
ANGLE OF DRIVE Y		60°-90°	60°-90°		60°-90°	60°-90°
DIVISIONAL ISLAND (Throat Median)		4'-22' Wide	4'-22' Wide		4'-22' Wide	4'-22' Wide
SETBACK G	12' Min., All See Genera	' categories. I Note No. 5.				

Z Side road intersection design, with possible auxiliary lanes and channelization, may be necessary. Intersection design, with possible auxiliary lanes and channelization, should be considered for connections with more than 4000 trips/days.

□ "2-Way" refers to one "in" movement and one "out" movement i.e., not exclusive left or right turn lanes on the connection.

🔅 When more than 2 lanes in the turnout connection are required, the 36' max. width may be increased to relieve interference between entering and exiting traffic which adversely affects traffic flow. These cases require documented site specific study and design.

riangle Small radii may be used in lieu of flares as approved by the Department.

DESIGN NOTE: 1-Way connections will be designed to effectively eliminate unpermitted movements.

NOT INTENDED FOR FULL INTERSECTION DESIGN SUMMARY OF GEOMETRIC REQUIREMENTS FOR DRIVEWAY TURNOUTS

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DESCRIPTION:



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GENERAL NOTES

- - leaving the highway.

For connections that are intended to daily accommodate either multi-unit vehicles or single unit vehicles exceeding 30' in length, returns with 50' radii shall be used, unless otherwise called for in the plans or otherwise stipulated by permit. Where large numbers of multi-unit vehicles will use the connection, the connection width and radii shall be increased and auxiliary lanes, tapers, lane flares, separators and/or islands constructed, as determined by the Department to be necessary for safe turning movements.

DESIGN NOTES

1. For definitions and descriptions of access connection "Categories" and access "Classifications" of highway segments, and for other detailed information on access to the State Highway System, refer to FDOT Rule Chapter 14-96, "State Highway Connection Permits Administrative Process" and Rule Chapter 14-97, "State Highway System Access Management Classification System And Standards."

2. For this index the term 'turnout' applies to that portion of driveways or side roads adjoining the outer roadway. For this index the term 'connection' encompasses a driveway or side road and their appurtenant islands, separators, transition tapers, auxiliary lanes, travelway flares, drainage pipes and structures, crossovers, sidewalks, curb cut ramps, signing, pavement marking, required signalization. maintenance of traffic or other means of access to or from controlled access facilities. The turnout requirements set forth in this index do not provide complete intersection design, construction or maintenance requirements.

3. The location, positioning, orientation, spacing and number of connections and median openings shall be in conformance with FDOT Rule Chapter 14-97.

Distance Between 4. On Department construction projects all driveways not shown on the plans shall be reconstructed at their existing location in conformance to these standards, or, in conformance to permits issued during the construction project.

> 5. Driveways shall have sufficient length and size for all vehicular queueing, stacking, maneuvering, standing and parking to be carried out completely beyond the right of way line. Except for vehicles stopping to enter the highway, the turnout areas and drives within the right of way shall be used only for moving vehicles entering or

6. Connections with expected daily traffic over 4000 vpd shall be constructed as intersecting side roads. The design requirement of this index and that of the local government will be used to select appropriate connection widths, radii and intersection design, subject to the approval of the Department. For connections with expected daily traffic less than 4000 vpd, the Department will determine if a drop curb or radius returns are required in accordance with existing or planned connections. Where radius returns apply, the design requirements of this index and that of the local government will be used to select appropriate connection widths. radii and intersection design, subject to the approval of the Department.

7. Any connection requiring or having a specified median opening with left turn storage and served directly by that opening shall have radial returns.

8. Where a connection is intended to align with a connection across the highway, the through lanes shall align directly with the corresponding through lanes.

9. For new connections and for connections on all new construction and reconstruction projects, pavement materials and thicknesses shall meet the requirements applicable to either that detailed for "Curbed Roadway-Flared Turnouts", or, that described in "Table 515-1" for connections with radial returns and/or auxiliary lanes.

10. The responsibility for the cost of construction or alteration to an access connection shall be in accordance with FDOT Rule Chapter 14-96.

1. Prior to the adoption of FDOT Rules Chapters 14-96 and 14-97, connections to the State Highway System were defined and permitted by Classes. Connections have been redfined by Categories under Rule 14–96; and, the term "Class" has been applied to highway segments of the State Highway System as defined under Rule 14-97.

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⅓" Open Joints @ 10' Ctrs.; Every 5th Joint Shall Be 1/2"

- 3' Curb Transition From Full To Zero Height At End Of All Curb And Gutter Construction Except Where Connecting Existing Curb And Gutter. \triangle
- Full Height Curb Length Varies: 2' Min. Beyond Limits Of Sidewalk Curb Ramps -See Index 522-002. 4' Min. Back Of Sidewalk In Absence Of Sidewalk Curb Ramps. As Required For Connecting To Existing Curb. \triangle

Side Road Edge Of Pavement Extension Line (Future Edge Of Pavement Where Available In An Adopted Five Year Transportation Plan).

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DRIVE ENTRANCES NOTES:

- trips per day, or 5 trips per hour as approved by permit or by the Engineer, or when not itemized in the plans.
- shall be 30' from edge of travel way or at R/W line, whichever is less.
- 🛛 See "Summary Of Geometric Requirements For Turnouts" chart for return radii lengths and supplemental information.
- Return Radius Point or Flare Point.

MATERIAL TYPES AND THICKNESSES IN DRIVING AREAS FOR ALL CONNECTIONS

Courses	Matariala 🕥	Thickness (in.) 🛈	
Course Materiais Ø	Maleriais @	Connections 3	Roadway 🏽
Structural	Asphaltic Concrete	1"	1 ½"
Bases	Optional Base (See Spec. Section 285)	0.B.G. 1	0.B.G. 3

① Minimum thickness.

② All materials shall be approved by the Department prior to being placed.

- ③ Connection structure other than traffic lanes. See Notes 1 and 2 below.
- (I) Travel way flares (bypass lanes), auxiliary lanes serving more than a single connection, and all median crossovers including their auxiliary lanes and/or transition tapers. See Notes 1 and 2 below.

NOTES

- 1. The pavement should be structurally adequate to meet the expected traffic loads and should not be less than that shown above, except as approved by the Department for graded connections. Other Department-approved equivalent pavements may be used at the discretion of the Engineer.
- 2. Auxiliary lanes and their transition tapers shall be the same structure as the abutting travel way pavement thickness or any of the roadway structures tabulated above, whichever is thicker.
- 3. If an asphalt base course is used for a turnout, its thickness may be increased to match the edge of travel way pavement thickness in lieu of a separate structural course. 6" of Portland cement concrete will be acceptable in lieu of the asphalt base and structural courses. See Notes 4 and 5 below.
- 4. A structural course is required for flexible pavements when they are used for auxiliary lanes serving more than a single connection.
- 5. Connections paved with Portland cement concrete shall be Class NS concrete at least 6" thick. The Department may require greater thickness when called for in the plans or stipulated by permit. Materials and construction shall conform with FDOT Standard Specifications Sections 347, 350 and 522.
- 6. The Department may require other pavement criteria where local conditions warrant.

PAVEMENT STRUCTURE FOR TURNOUTS AND AUXILIARY LANES TABLE 515-1



NOTES

1. Auxiliary lane pavements and crossover pavements shall be

- 2. Department maintenance of turnout pavement extends 5' from the edge of paved shoulder, whichever is greater. The rem area on the right of way shall be maintained by the owner function of routinely reworking shoulders, the Department n material on nonpaved areas beyond the maintained pavement
- 3. Control and maintenance of drainage facilities within the rig responsibility of the Department, unless specified different
- 4. The maintenance and operation of highway lighting, traffic and other necessary devices shall be the responsibility of
- 5. All pavement markings on the State highways, including acce markings, and signing installed for the operation of the Sta by the Department.
- 6. All signing and marking installed for the operation of the co and stop signs for the connection) shall be the responsibilit

LIMITS OF CONSTRUCTION AND MAINTENANCE FOR FLUSH SHOULDER ROADWAY CONNECTIONS

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ST	NC	DESCRIPTION:
SION	SI	
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TURNOUTS AND DRIVEWAY

Edge of Travel Way

R/W						
	LARG	5IDE ROAE OR E COMMEF DRIVEWAY	D RCIAL			
 	uxiliary L See Note	.ane Width #1)				
R/W		ND Graded Or Pa Required Pav Limits Of Dec	ved ing partment N	Aaintenanc	e	
	→	Lane Identific	cation and	Direction	of Traffi	с
maintai	ned by th	ne Department.				
m edge nainder or his a may graa t.	of the tr of any tu authorized de and si	ravel way or to irnout paved d agent. As a hape existing)			
ght of w Iy by D	vay shall epartmen	be solely the t permit.				
signals, a public	associat agency.	ed equipment,				
eleration ate high	n and dec way shali	celeration lane I be maintained	1			
onnectic ty of the	on (such a e permitt	as stop bars ee.				

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(See Flush Shoulder Roadway – Turnout Profile, Left) -0.03 0.03 ROUNDED 0.02 0.02 0 5 10 0 0.02 0.02 5 10 10 6 0.02 0.02 10 7 0.02 0.00 10 8 ≠0.⁰⁶ 12 9 13 10 G₁ =0.05 0.02 14 11 $G_1 = 0.04$ 0.03 16 12 16 12.5 $G_1 = 0.03$ 0.04 17 13 $G_1 = 0.02$ 13.5 18 0.05 18 14 $G_1 = 0.01$ 0.06 19 14.5 20 15 $G_1 = 0.00$ 0.06 21 17 $G_1 = 0.01$ 22 18 0.06 $G_1 = 0.02$ 23 20 0.06 25 21 $G_{I} = 0.03$ 26 22 0.06 $G_1 = 0.04$ 28 24 29 25 0.06 G, =0.05 31 26 $G_{1} = 0.06$ 0.06 27 32 34 28 =0.07 0.06 35 30 36 31 0.07 0.09 0.10 ROADWAY PAVEMENT SLOPES AND SLOPES OF ABUTTING FLUSH SHOULDER ROADWAY TURNOUT SURFACES (G_2) SUPERELEVATION SECTIONS SHEET INDEX 000-515 7 of 7

G2 Slopes



C/ EC/ 07

	int *	4' Shoulder Pavement	Shoulder Slope Break - 	<u>\</u>	
					360
- <u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u>	\				Varies
	<u>+</u>	10-1			
		/4 ±	\sim		
♦ 8' Shoulder Pavement	- 6' Pavt			Ramp Width	10' Shoulder Pavt.
Shoulder Slope Break		Type Same As Ramp Pavt. /			4
		Shoulder Gutter 🗸	Concrete Pavement Or Flexible Particular	vement Varies	
			350'	Samp	₿ Ramp
	Shoulder Pavement Tun				
		e Same As Ramp Pavement		-	
				F	H lexible Pavement Thickness Tran
			TWO THRU LAN	IES	=100' (See DETAIL "A", Sheet 2)
			(Shown With Shoulder	Gutter)	
Concrete Pavement Or Flexible Paveme	ent 4	— 4' Shoulder Pavement	Shoulder Slope Break -	\	725' - 12' Lane 4' Shoulder Paveme
				_	
<u>3 LANES</u>	\downarrow <u>2 LANES</u> <u>12'</u>		4,	×	
)4°±			
10' Shoulder Pavement				Ramp Width	
Shoulder	r Slope Break ∕	Concrete Pavement Or Flexible Pav	rement		N 4 10
			-	1 10	
	-	Sad This	350'	Ram	₿ Ramp
		500 This Area From Edge Of Si	houlder Pavement To Ditch Bottom	<u>ر</u> ها	
			Dictin Bollom		
					L=100' (See DETAIL "A", Sheet 2
				WO THRU LANES	
		/ I	Ίκεε ΑΡΡΚΟΆζΠ LANES – Π	mo mo max	
		/ /	(Shown Without Shoulde	r Gutter)	
GENERAL NOTES:		I I	(Shown Without Shoulde	r Gutter)	
GENERAL NOTES: 1. Taper-Type exit and entrance terminals For such ramps, parallel deceleration a	as detailed shall not be u nd acceleration lanes shall	sed on ramps for which a speed of 50 be used in place of tapers with length	MPH or greater cannot be maintained. s set according to AASHTO.	r Gutter)	
GENERAL NOTES: 1. Taper-Type exit and entrance terminals For such ramps, parallel deceleration and 2. Shoulder Pavement:	as detailed shall not be u nd acceleration lanes shall	sed on ramps for which a speed of 50 I be used in place of tapers with length	MPH or greater cannot be maintained. s set according to AASHTO.	r Gutter)	
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GENERAL NOTES: 1. Taper-Type exit and entrance terminals For such ramps, parallel deceleration a. 2. Shoulder Pavement: A. Concrete Pavement Projects: Where roadway pavement beginning with th B. Flexible Pavement Projects: Where identical to the adjacent roadway p 3. For concrete pavement joint details and	as detailed shall not be u and acceleration lanes shall e shoulder pavement adjace he transverse joint neares shoulder pavement used in oavement. ' layouts at entrance and e	sed on ramps for which a speed of 50 I be used in place of tapers with length ent to shoulder gutter is less than 6' wh it the point of 6' width. In conjunction with shoulder gutter is le exit ramp terminals, see Index 350-001.	MPH or greater cannot be maintained. s set according to AASHTO. de, it shall be identical to the adjacent ss than 6' uniform width, it shall be	r Gutter)	
GENERAL NOTES: 1. Taper-Type exit and entrance terminals For such ramps, parallel deceleration a. 2. Shoulder Pavement: A. Concrete Pavement Projects: Where roadway pavement beginning with tl B. Flexible Pavement Projects: Where identical to the adjacent roadway p 3. For concrete pavement joint details and	as detailed shall not be u and acceleration lanes shall e shoulder pavement adjace he transverse joint neares shoulder pavement used in pavement. I layouts at entrance and e	sed on ramps for which a speed of 50 I be used in place of tapers with length ent to shoulder gutter is less than 6' wh it the point of 6' width. In conjunction with shoulder gutter is le exit ramp terminals, see Index 350-001.	MPH or greater cannot be maintained. s set according to AASHTO. de, it shall be identical to the adjacent ss than 6' uniform width, it shall be	r Gutter)	SII
GENERAL NOTES: 1. Taper-Type exit and entrance terminals For such ramps, parallel deceleration a. 2. Shoulder Pavement: A. Concrete Pavement Projects: Where roadway pavement beginning with th B. Flexible Pavement Projects: Where identical to the adjacent roadway p 3. For concrete pavement joint details and LAST VISION	as detailed shall not be u and acceleration lanes shall e shoulder pavement adjace he transverse joint neares shoulder pavement used ir pavement. I layouts at entrance and e	sed on ramps for which a speed of 50 I be used in place of tapers with length ent to shoulder gutter is less than 6' wh it the point of 6' width. In conjunction with shoulder gutter is le exit ramp terminals, see Index 350-001.	MPH or greater cannot be maintained. s set according to AASHTO. de, it shall be identical to the adjacent ss than 6' uniform width, it shall be	r Gutter)	511











on Course 8" Into Shoulder				
Extend Friction Course 8" Into	Sh <u>ould</u> er			
	10' Shoulder F	Pavement		
on Course 8" Into Shoulder				
tend Friction Course 8" Into Shc	oulder			
	8' Shoulder	Pavement		
LEGEND:				
E	xclude Friction	Course		
	riction course (prionar		
ATION (FOR FLEXIBLE PAVEMENT)				
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