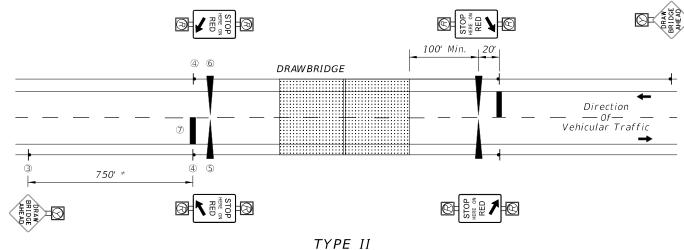


TO BE USED WHERE BRIDGE

OPERATORS ARE FULL TIME OR A DAILY BASIS.



TO BE USED WHERE TYPE I IS NOT

APPLICABLE (USUALLY WHEN THE BRIDGE OPERATOR IS "ON CALL").

LEGEND:

- ① TRAFFIC SIGNALS) Mast Arm Mounted (Off Bridge)
- ② DRAWBRIDGE SIGN J Monotube Support Mounted (On Bridge)
- DRAWBRIDGE AHEAD SIGN WITH YELLOW FLASHING BEACON
- STOP HERE ON RED SIGN WITH RED FLASHING BEACONS
- ENTRANCE GATE
- EXIT GATE
- 24" THERMOPLASTIC STOP BAR

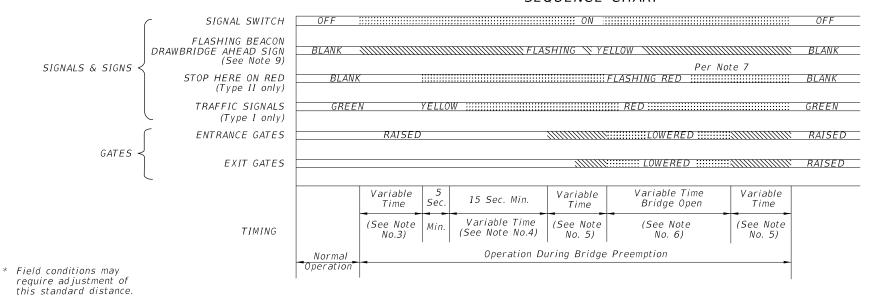


SLIPPERY WHEN WET SIGN See Note 11

NOTES:

- 1. A bypass switch shall be installed to override each timing interval in case of a malfunction
- 2. "STOP HERE ON RED" is omitted in Type I operation and "TRAFFIC SIGNALS" are omitted in Type II operation.
- 3. The time between beginning of flashing yellow on "Drawbridge Ahead" sign and the clearance of traffic signal to red, or beginning of flashing red should not be less than the travel time of a passenger car, from the sign location to the stop line, traveling at the 85 percentile
- 4. Beginning of operation of drawbridge gates shall not be less than 15 seconds after steady red or 20 seconds after flashing red (Actual time may be determined by the bridge tender.)
- 5. Time of gate lowering and raising is dependent upon gate type.
- 6. Time of bridge opening is determined by the bridge tender
- 7. Each gate shall be operated by a separate switch.
- 8. On each approach (Type II), all four red signals shall be on the same two circuit flashers, with the two top signals on one circuit, and the two bottom signals on the alternately flashing
- 9. A Drawbridge Ahead sign is required for both types of signal operation, However a flashing beacon shall be added to the sign when physical conditions prevent a driver traveling at the 85% approach speed from having continuous view of at least one signal indication for approximately 10 seconds.
- 10. Requirements on gate installation are contained in Section 4I of the "Manual on Uniform Traffic Control Devices".
- 11. "In accordance with Traffic Engineering Manual (Topic Number 750-000-005) Section 2.1 SLIPPERY WHEN WET SIGNS shall be placed in advance of all MOVABLE and NONMOVABLE STEEL DECK BRIDGES."

SEQUENCE CHART



Ground Mounted

LAST **REVISION** 11/01/17

FDOT

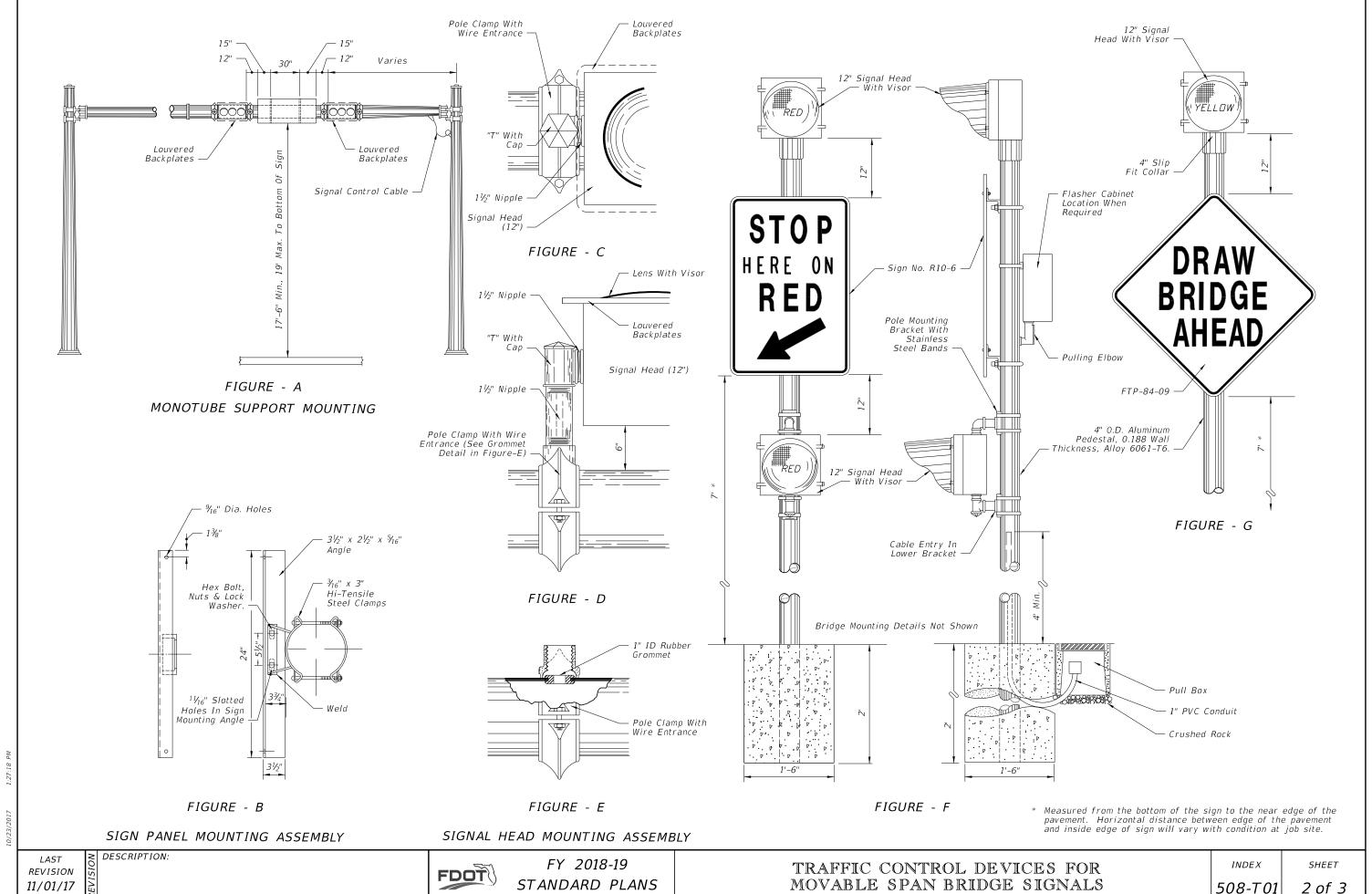
FY 2018-19 STANDARD PLANS

TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS

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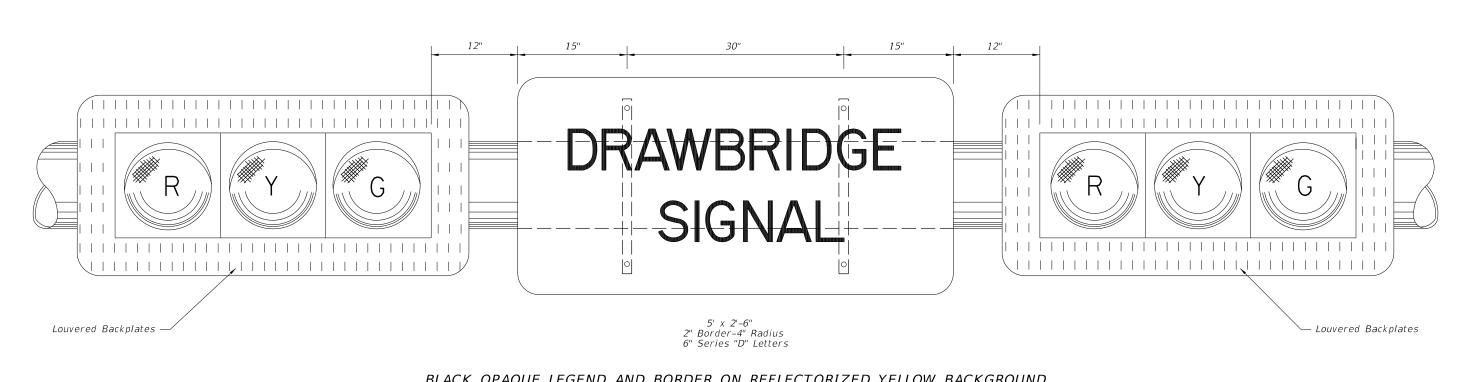
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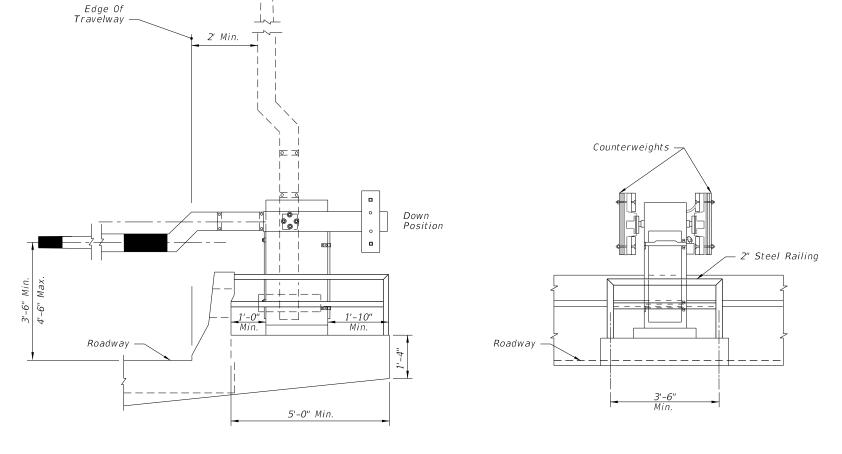
STANDARD PLANS

MOVABLE SPAN BRIDGE SIGNALS

2 of 3



BLACK OPAQUE LEGEND AND BORDER ON REFLECTORIZED YELLOW BACKGROUND TO BE USED WITH TYPE I OPERATION, AS SHOWN ON PREVIOUS SHEET MONOTUBE SUPPORT MOUNTING



Class I Or II (Length Shall Be Shown On Plan Sheets) RR & Drawbridge Arms 18' to 20' Center Line Mast RR & Drawbridge 2'-10" Center 6'-0" Arms 32' And Over

NOTES:

- 1. 12 volt flashing red lights shall be mounted on gate arm and shall operate in the flashing mode only when gate arm is in the lower position or in the process of being lowered. The number of lights shall vary accordingly to length of the gate arm.
- 2. Alternating 16" pattern of fully reflectorized red and white stripes.

GATE & ARM DETAIL

TYPICAL LAMP PLACEMENT

REVISION 11/01/17

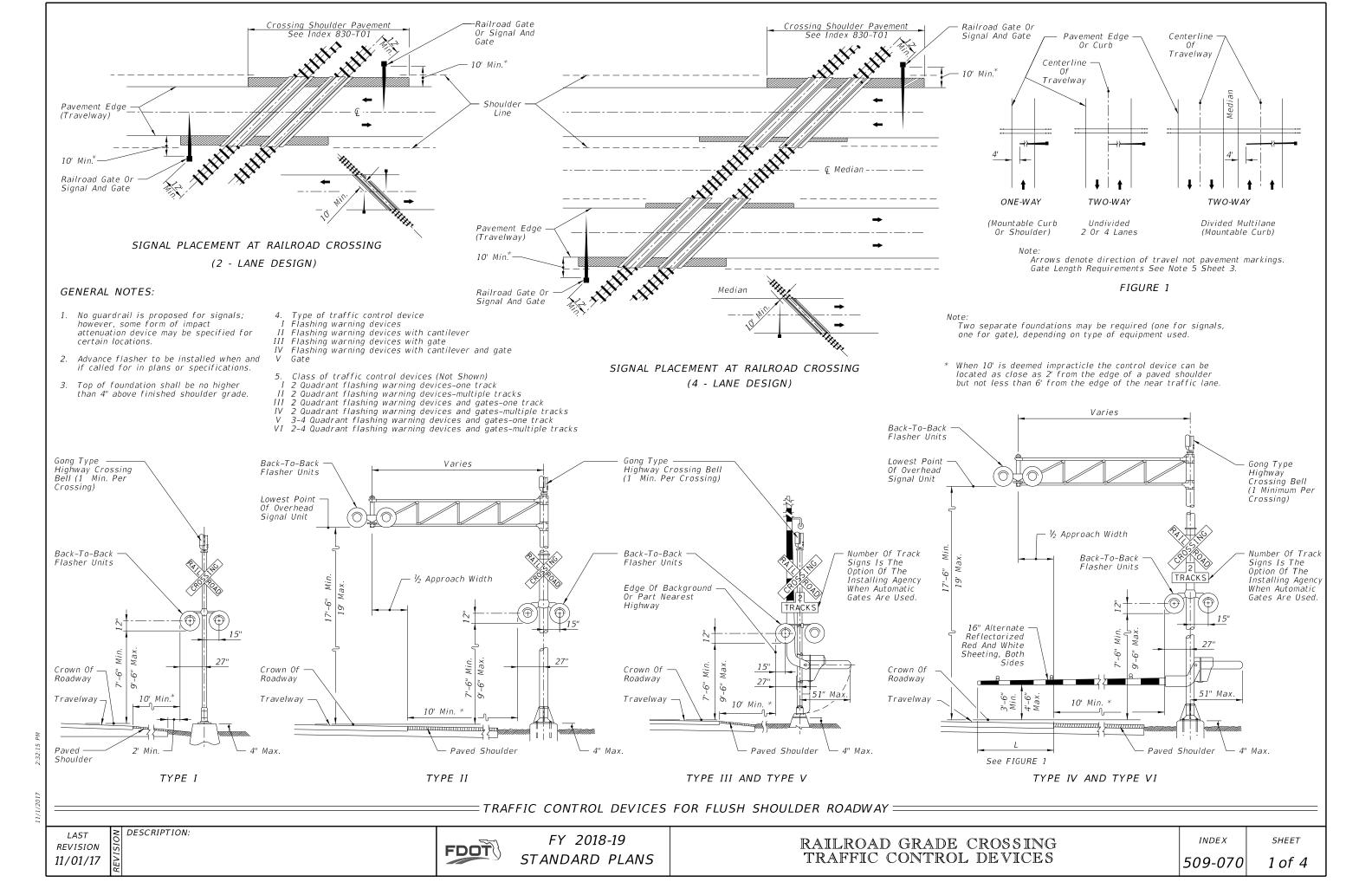
FDOT

FY 2018-19 STANDARD PLANS

TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS INDEX

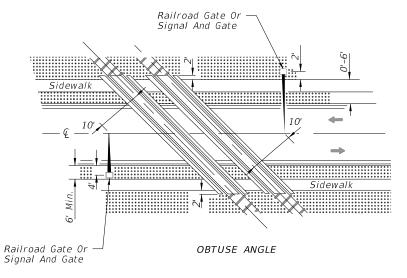
SHEET

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Railroad Gate Or Signal And Gate Railroad Gate Or Signal And Gate ACUTE ANGLE (AND RIGHT ANGLE)

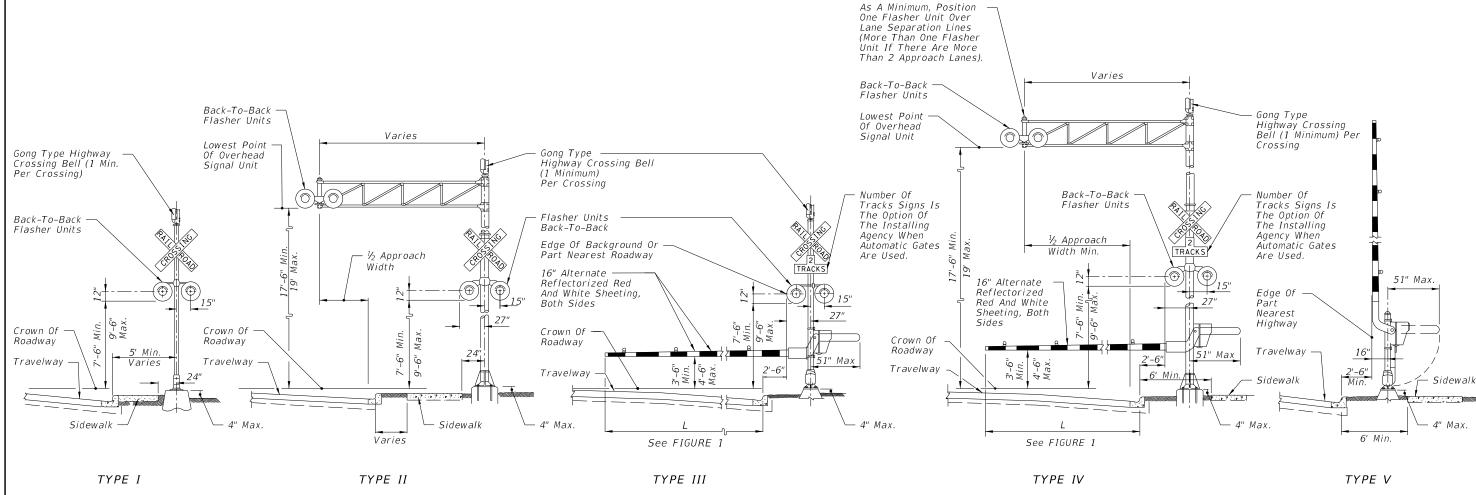
SIGNAL PLACEMENT AT RAILROAD CROSSING
(2 LANES, CURB & GUTTER)



SIGNAL PLACEMENT AT RAILROAD CROSSING
(2 LANES, CURB & GUTTER)

NOTES:

- 1. The location of flashing warning devices and stop lines shall be established based on future (or present) installation of gate with appropriate track clearances.
- 2. Where plans call for railroad traffic control devices to be installed in curbed medians, the minimum median width shall be 12'-6".
- 3. Location of railroad traffic control device is based on the distance available between face of curb & sidewalk. O' to 6' Locate device outside sidewalk. Over 6' Locate device between face of curb and sidewalk.
- 4. Stop line to be perpendicular to edge of roadway, approx. 15' from nearest rail; or 8' from and parallel to gate when present.
- 5. When a cantilevered-arm flashing warning device is used, the minimum vertical clearance shall be 17'-6" from above the Crown of Roadway to the Lowest Point of the Overhead Signal Unit.



TRAFFIC CONTROL DEVICES FOR CURBED ROADWAY

11/1/2017

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES

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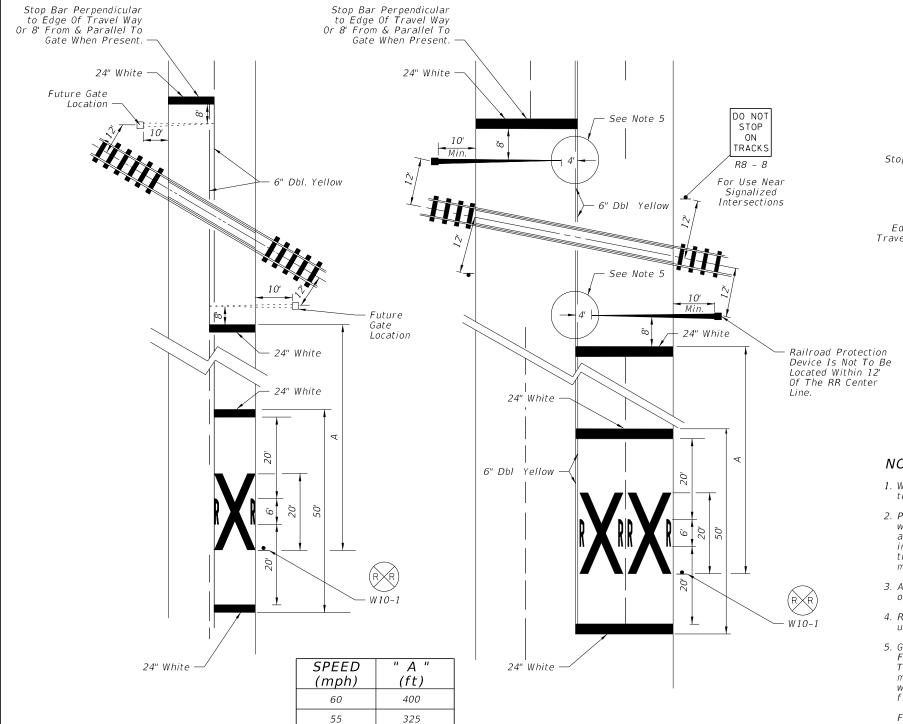
SHEET

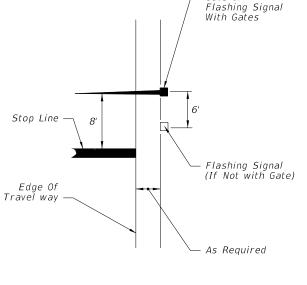
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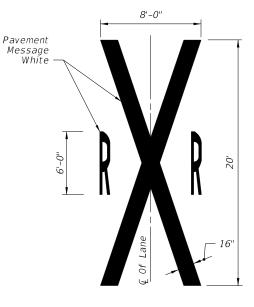
RAILROAD CROSSING AT TWO (2)-LANE ROADWAY

RAILROAD CROSSING AT MULTILANE ROADWAY

RELATIVE LOCATION OF CROSSING TRAFFIC CONTROL DEVICES



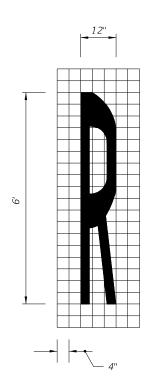




NOTES:

- 1. When computing pavement message, quantities do not include
- 2. Placement of sign W10-1 in a residential or business district, where low speeds are prevalent, the W10-1 sign may be placed a minimum distance of 100' from the crossing. Where street intersections occur between the RR pavement message and the tracks an additional W10-1 sign and additional pavement message should be used.
- 3. A portion of the pavement markings symbol should be directly opposite the W10-1 sign.
- 4. Recommended location for FTP-61-06 or FTP-62-06 signs, 100' urban and 300' rural. See Index 700-102 for sign details.
- 5. Gate Length Requirements: For Two-way undivided sections:
 The gate should extend to within 1' of the center line. On multiple approaches the maximum gate length may not reach to within 1' of the center line. For those cases, the distance from the gate to the center line shall be a maximum of 4'.

For one-way or divided sections: The gate shall be of sufficient length such that the distance from the gate tip to the inside edge of pavement is a maximum



REVISION 11/01/17

FDOT

50

45

40

35

URBAN

250

175

125

100

85 MIN.

FY 2018-19 STANDARD PLANS

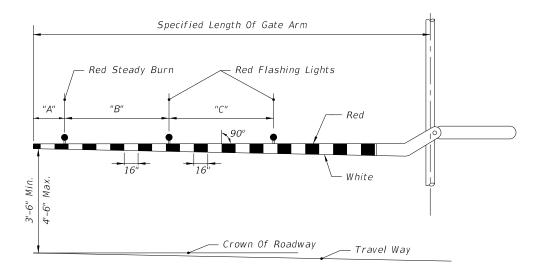
RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES

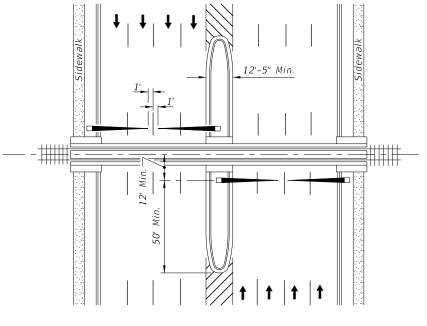
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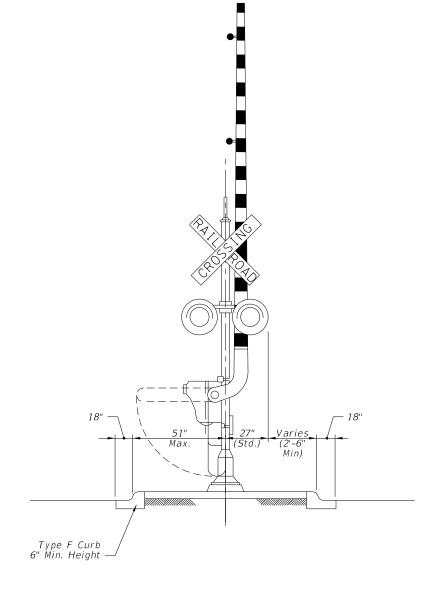
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DESCRIPTION:





PLAN



MEDIAN SECTION AT SIGNAL GATES

RAILROAD GATE ARM LIGHT SPACING

Specified Length Of Gate Arm	Dimension "A"	Dimension "B"	Dimension "C"
14 Ft.	6"	36"	5'
15 Ft.	18"	36"	5′
16-17 Ft.	24"	36"	5′
18-19 Ft.	28"	41"	5'
20-23 Ft.	28"	4'	5'
24-28 Ft.	28"	5'	5'
29-31 Ft.	36"	6'	6'
32-34 Ft.	36"	7'	7'
35-37 Ft.	36"	9'	9'
38 And Over	36"	10'	10'

NOTE: For additional information see the "Manual On Uniform Traffic Control Devices", Part 8; The "Traffic Control Handbook" , Part VIII; and AASHTO "A Policy On Geometric Design Of Streets And Highways".

MEDIAN SIGNAL GATES FOR

MULTILANE UNDIVIDED URBAN SECTIONS

(THREE OR MORE DRIVING LANES IN ONE DIRECTION, 45 MPH OR LESS)

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2018-19 STANDARD PLANS RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES

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3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS						
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS			
Post "A"	HSS 2½ x 1½ x⅓	2.50" x 1.50"	0.125"			
Post "B"	HSS 2½ x 1½ x¾6	2.50" x 1.50"	0.188"			
Top Rail	2½" NPS (Sch. 10)	2.875"	0.120"			
TOP KAIT	HSS 3.000 x 0.120	3.000"	0.120"			
End Hoops	2½" NPS (Sch. 10)	2.875"	0.120"			
	HSS 3.000 x 0.120	3.000"	0.120"			
Top Rail Joint/Splice Sleeves	HSS 2.500 x 0.125	2.500"	0.125"			
Intermediate & Bottom Rail	HSS 2 x 2 x 3/16	2.00" x 2.00"	0.188" ⁽¹⁾			
Int. & Bottom Rail Post Connection Sleeve	HSS 1.500 x 0.125	1.500"	0.125" ⁽¹⁾			
Handrail laint/Calina Classes	1" NPS (Sch. 40)	1.315"	0.133"			
Handrail Joint/Splice Sleeves	HSS 1.500 x 0.125	1.500"	0.125"			
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrail Support Bar	¾" Ø Round Bar	0.750"	N/A			
Pickets (Type 1 Infill Panel)	³¼" Ø Round Bar	0.750"	N/A			
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies			

TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be $1\frac{1}{4}$ " NPS (Sch. 40).

NOTES =

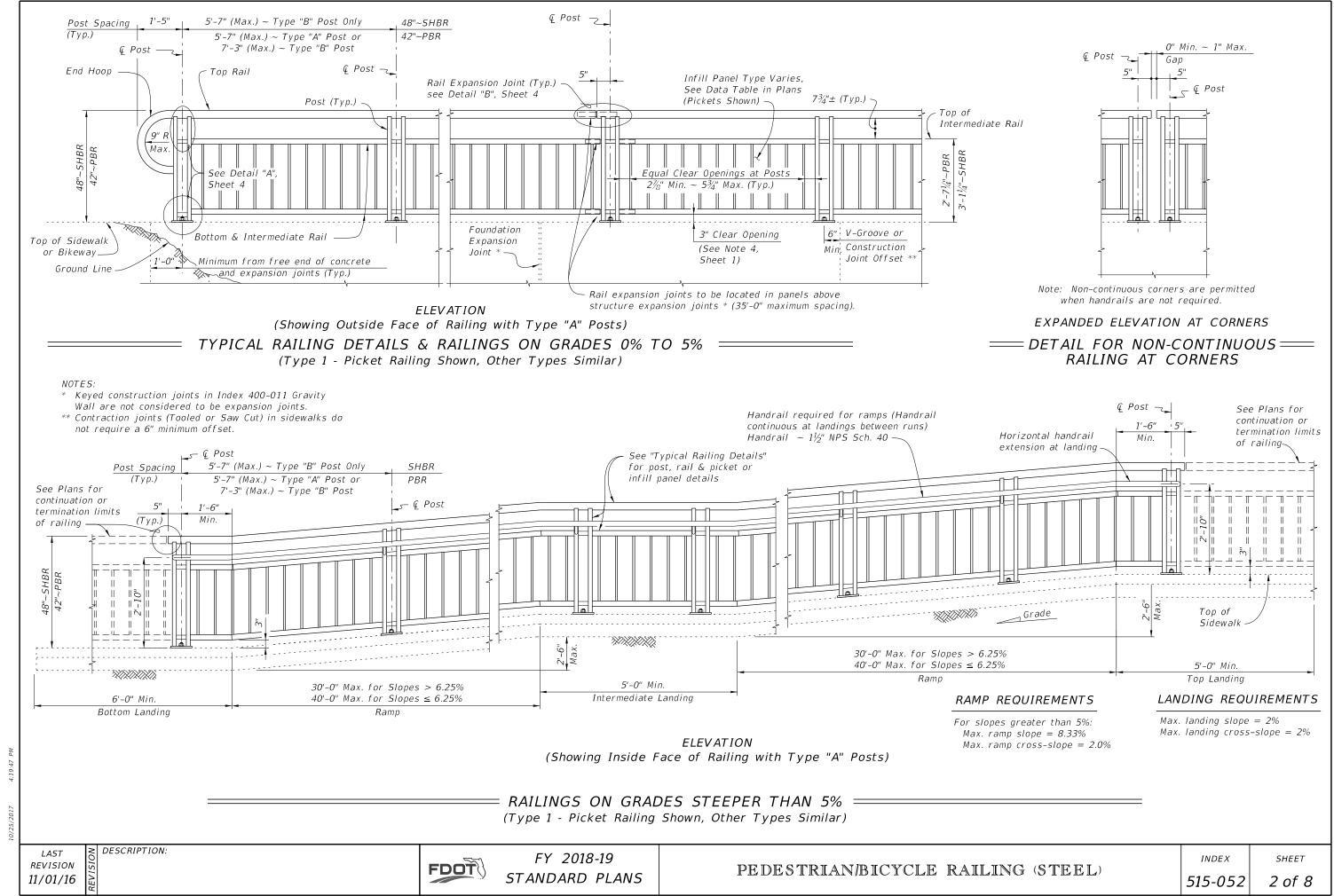
- 1. Shop Drawings are required; see Specification Section 515
- 2. For bridge mounted railings work this Index with Index 515-051 Bridge Bicycle/Pedestrian Railing
- - A. Pipe Rails and Pickets: ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) and ASTM A36 for bars.
 - B. Structural Tube: ASTM A500 Grade A, B, C, or D or ASTM A501
 - C. Steel Plate: ASTM A36 or ASTM A709 Grade 36
 - D. U-Channels and filler plates: ASTM A36 or ASTM A1011 (Grade 36).
 - E. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
 - F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962.
 - a. Hex Head Bolts: ASTM A 307 or ASTM F1554
 - 1. $\frac{1}{8}$ " diameter single bolt option, Grade 36
 - 2. \(\gamma_6''\) four bolt option, Grade 55
 - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
 - c. Hex Nuts: ASTM A563
 - d. Flat Washers: ASTM F436
 - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
 - G. Shims: ASTM B209 Alloy 6061
 - H. Bearing Pads: 1/8" Plain, Fabric Reinforced or Fabric Laminated pads that meet the requirements of Specification Section 962 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5% for standard installations and 3% when a 4" sphere requirement is indicated in the Data Tables.
- 5. Maximum spacing between expansion joints is 40'-0". Locate an Expansion Joint between the posts on either side of the Deck
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K".
- 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for:
 - A. Grades Steeper than 5%,
 - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

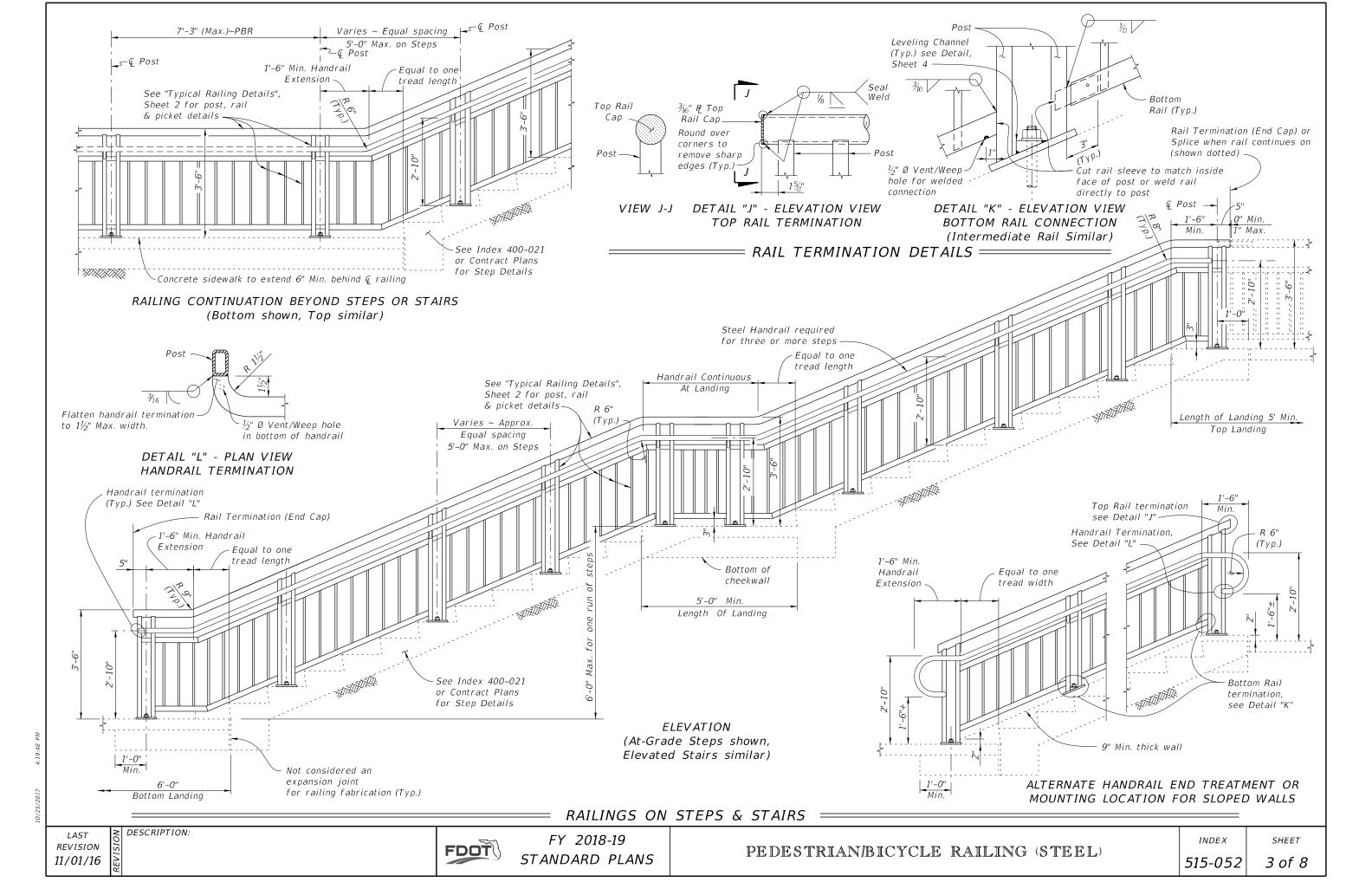
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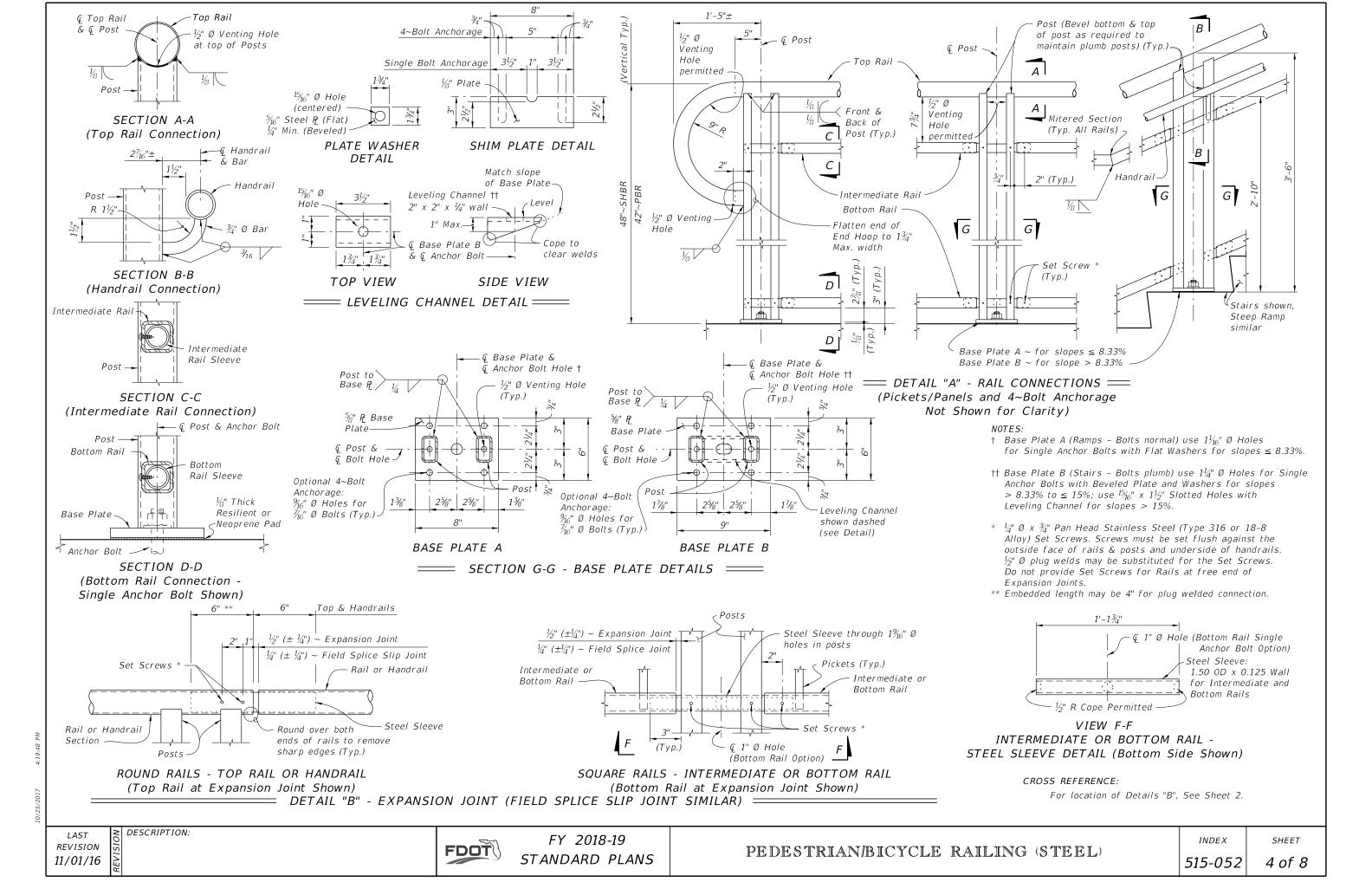


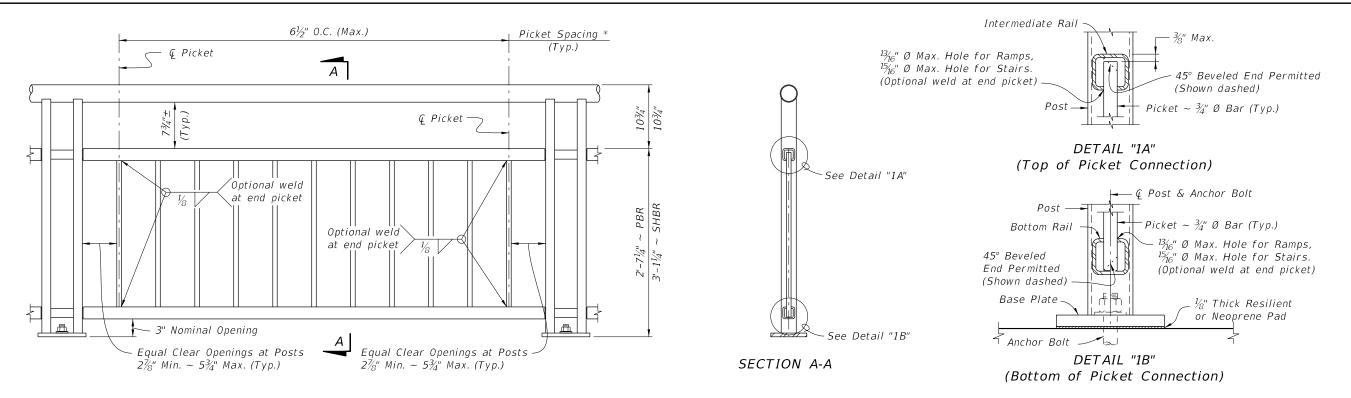
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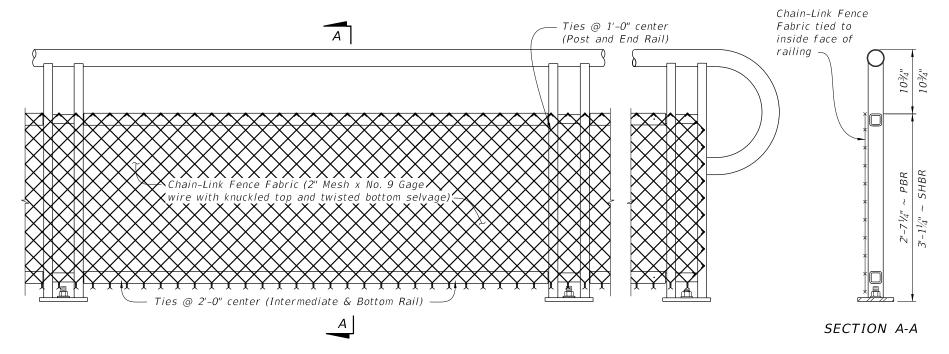




TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. See Note 4 (Sheet 1).



Components	F 6
CHAIN-LINK PAN	IEL NOTE:

COMPONENT

Chain-Link Fence

Fabric (2" mesh with

knuckled top selvage)

twisted bottom and

Tie Wires

Tension Bars

Miscellaneous Fence

Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS

COMPONENT INFORMATION

Polyvinyl Chloride (PVC) Coated Steel - No. 9 gage Zinc-Coated Wire (metallic-coated

Zinc-Coated Steel Wire - No. 9 gage with

coating to match Chain-Link Fence Fabric. $\frac{3}{16}$ " (Min. thickness) x $\frac{3}{4}$ " (Min. width)

Zinc-Coated Steel - No. 9 gage (coated

Aluminum-Coated Steel - No. 9 gage

core wire diameter) ~ See Plans for

x 2'-3' (Min. height) Steel Bars

wire diameter), Class 2 Coating

(coated wire diameter)

specified color of PVC.

Zinc-Coated Steel

ASTM

A 392

A 491

F 668

F 626

F 626

TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES:

1. See Plans for Infill Panel option required.

DESCRIPTION: **REVISION** 11/01/16

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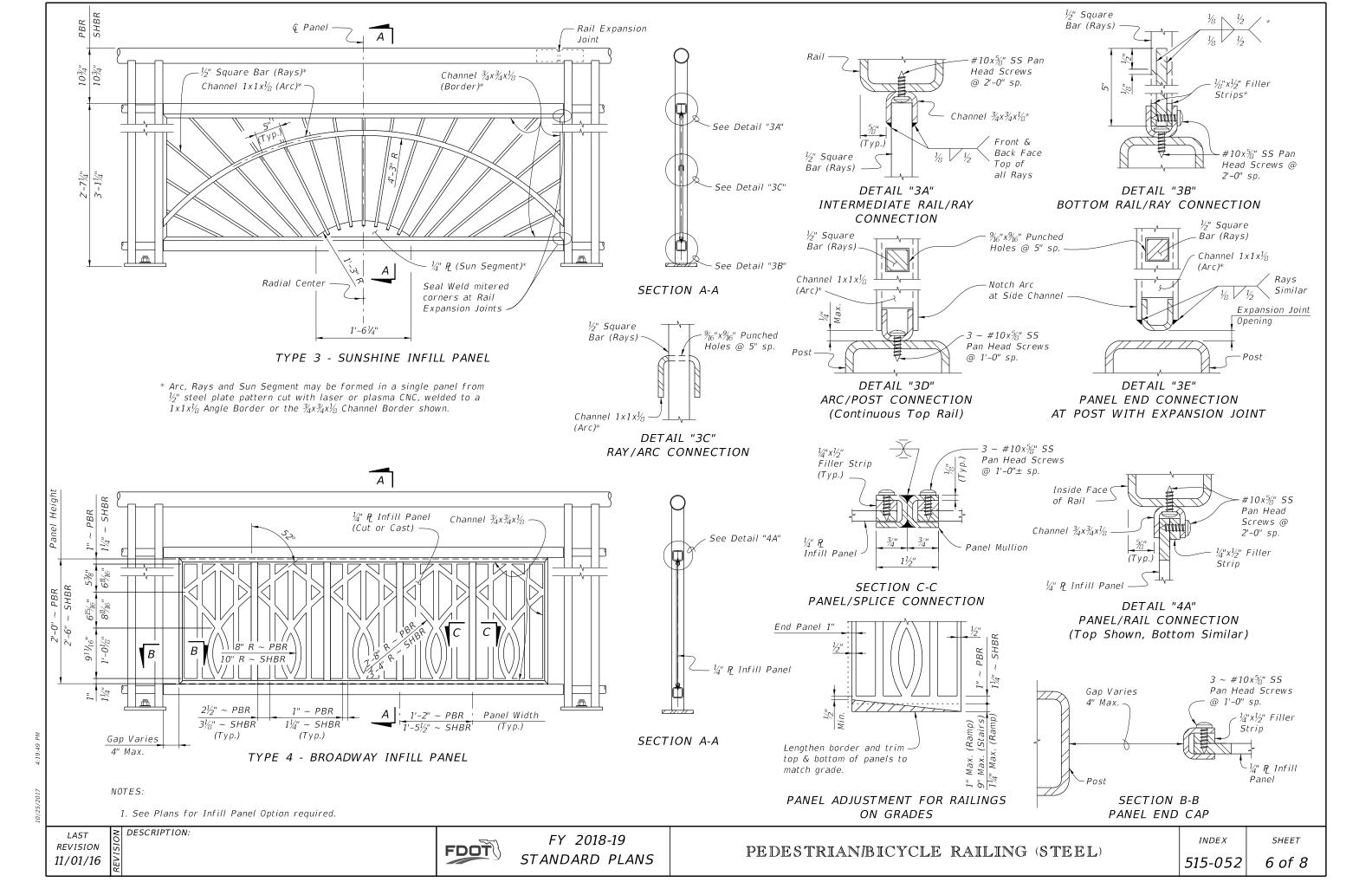
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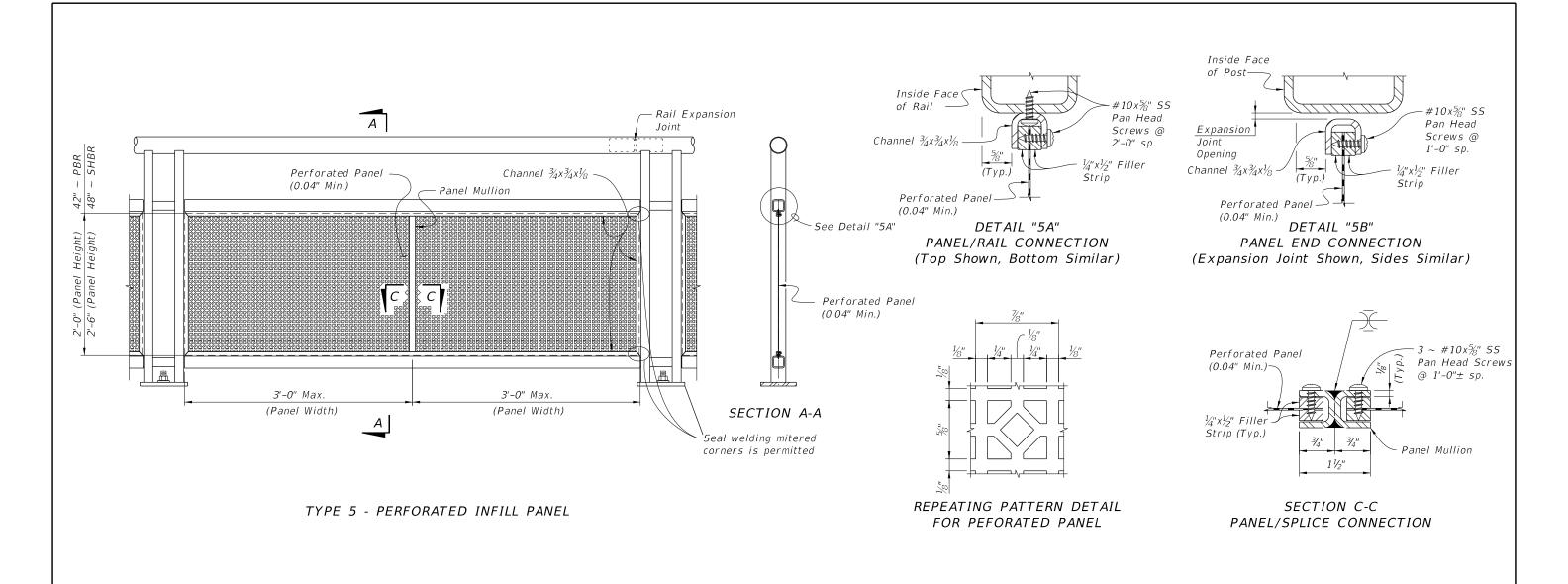
PEDESTRIAN/BICYCLE RAILING (STEEL)

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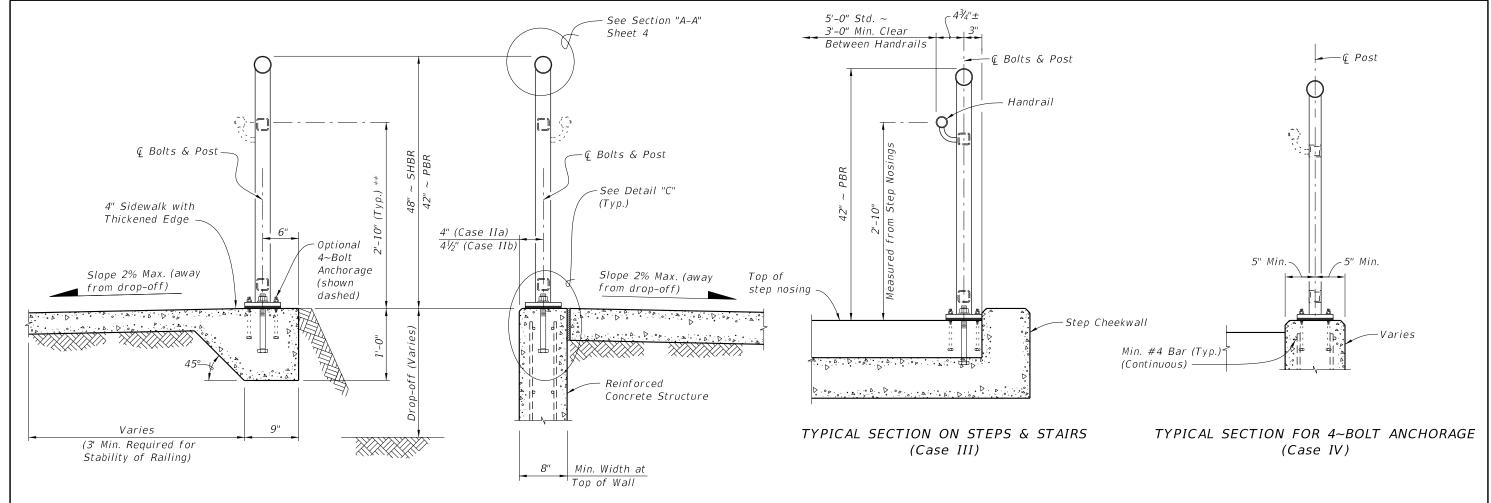
NOTES:

DESCRIPTION:

1. See Plans for Infill Panel Type required.

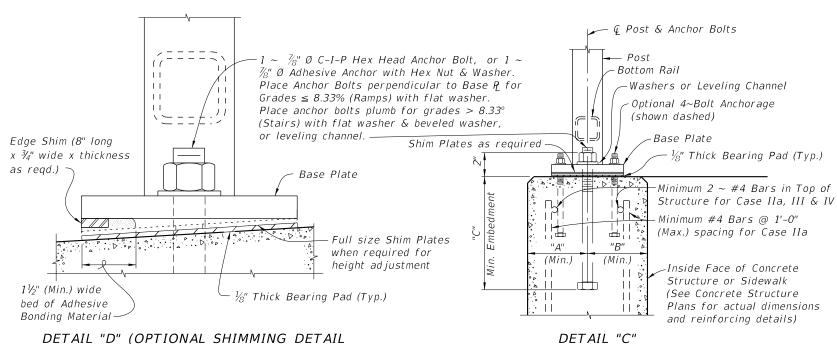
REVISION 11/01/16

FDOT



TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

TYPICAL SECTION ON RETAINING WALL (Case II)



	ANCHOR BOLT TABLE							
CACE		DIMENSIONS			ANCHOR LENGTH		ANGUOD	
CASE	STRUCTURE TYPE	A Edge Dist.	B Edge Dist.	C Embedment	C-I-P Hex Head Bolt	Adhesive Anchor	ANCHOR SIZE	
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	½" Ø	
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø	
IIb	Gravity Wall Index 400-011	41/2"	3½" @ top	1'-0" *	1'-1½"	1'-2"	%" Ø	
III	Step Cheekwall	4 ¹ / ₂ "	4½"	9"	10½"	11"	%" Ø	
IV	Varies	5"	5"	5"	6½"	7"	%6" Ø	

- * Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".
- ** When required; measured from top of sidewalk.

DETAIL "C" FOR CROSS SLOPE CORRECTION) (Cast-In-Place Anchor Bolts shown, (Used in lieu of Beveled Shim Plates) Adhesive Anchors similar)

REVISION 11/01/16

DESCRIPTION:

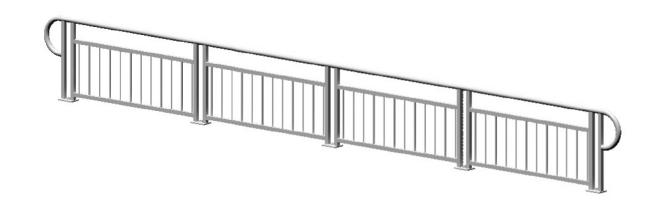
FDOT

FY 2018-19 STANDARD PLANS

PEDESTRIAN/BICYCLE RAILING (STEEL)

INDEX 515-052

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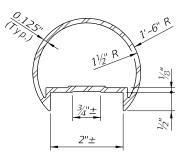


3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS					
MEMBER	ALLOY ⁽¹⁾	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS	
Posts (Type "A" & "B")	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250"	
Posts (Type "C")	6061-T6	Extrusion 1½x2½x0.125	1.50" x 2.50"	0.125"	
Top Plate (Type "C")	6061-T6	Extrusion (See Details)	2¾" x 7"	Varies	
Ton Dail	COC1 TC	2½" NPS (Sch. 10)	2.875"	0.120"	
Top Rail	6061-T6	3" Round Top Cap Rail	3.000"	0.125"	
End Hoops	6063-T5	2½" NPS (Sch. 10)	2.875"	0.120"	
		3.00 OD x 0.125 Wall	3.000"	0.125"	
	6063-T5	2.50 OD x 0.125 Wall	2.500"	0.125"	
Top Rail Joint/Splice Sleeves		Top Cap Rail Inner Sleeve	2.800"	0.090"	
Intermediate & Bottom Rail	6061-T6	RT 2x2x0.250	2.00" x 2.00"	0.250" (2)	
Int. & Bottom Rail Post Connection Sleeve	6063-T5	1.50 OD x 0.125 Wall ⁽³⁾	1.500"	0.125"	
	6063-T5	1" NPS (Sch. 40)	1.315"	0.133"	
Handrail Joint/Splice Sleeves	6063-T5	1.50 OD x 0.125 Wall	1.500"	0.125"	
Handrails	6061-T6	1½" NPS (Sch. 40)	1.900"	0.145"	
Handrail Support Bar	6061-T6	3/4" Ø Round Bar	0.750"	N/A	
Pickets (Type 1 Infill Panel)	6061-T6	¾" Ø Round Bar	0.750"	N/A	
Infill Panel Members (Types 2 - 5)	6063-T5	Varies (See Details)	Varies	Varies	

TABLE 1 NOTES:

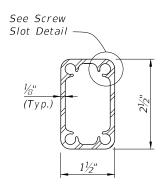
- (1) Alloy 6061-T6 or 6063-T52 & T6 may be substituted for Alloy 6063-T5.
- (2) 0.188" wall thickness permitted for rails with post spacings less than 5'-9".
- (3) 1" NPS (Sch. 40) non-slit rail sleeves may be substituted when welded connection Detail "K" is utilized.



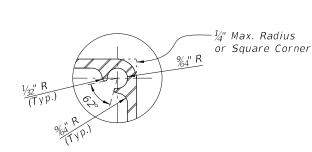
3" ROUND TOP CAP RAIL TOP CAP RAIL INNER SPLICE SLEEVE



ALTERNATIVE BOTTOM & INTERMEDIATE RAIL SECTION FOR TYPE 3, 4 & 5 RAILINGS



POST TYPE "C" SCREW SLOT SECTION



SCREW SLOT DETAIL

F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962. a. Hex Head Bolts: ASTM A 307

A. Structural Extrusions, Tube, Pipe and Bars: Table 1 and ASTM B221 or ASTM B429

1. Shop Drawings are required, see Specification Section 515.

B. Base Plates and Rail Caps: ASTM B209 Alloy 6061-T6

D. Stainless steel (SS) screws: Type 316 or 18-8 Alloy E. Aluminum screws: Alloy 2024-T4 or 7075-T73

- 1. %" diameter single bolt option, Grade 36
- 2. $\frac{7}{16}$ " diameter four bolt option, Grade 55
- b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
- c. Hex Nuts: ASTM A563

3. Materials:

d. Flat Washers: ASTM F436

C. Perforated panels (Type 5) Alloy 3003-H14

- e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
- G. Shims: ASTM B209 Alloy 6061 or 6063
- H. Bearing Pads: Provide $\frac{1}{2}$ " thick Plain, Fabric Reinforced or Fabric Laminated Bearing Pads meeting the requirements of Specification Section 962 for Ancillary Structures.

NOTES:

a. Top, bottom and intermediate rail corner bends with maximum 4'-0" post spacing may be Alloy 6063-T6

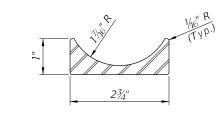
2. For bridge mounted railings, work this Index with Index 515-061 Bridge Bicycle/Pedestrian Railing (Aluminum)

- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5%" for standard installations and 3%" when a 4" sphere requirement is indicated in the Data Tables.
- 5. Locate railing expansion Joints between the posts on either side of
- the deck expansion joint. Maximum spacing between expansion joints is 35'-0".
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K" for Post Type "A" & "B".
- 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for:
 - A. Grades Steeper than 5%,
 - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

CROSS REFERENCES:

Detail "A", Sheet 4 Detail "B", Sheet 4

Detail "K", Sheet 3



OPTIONAL TOP PLATE EXTRUSION SECTION (POST TYPE "C")

DESCRIPTION:

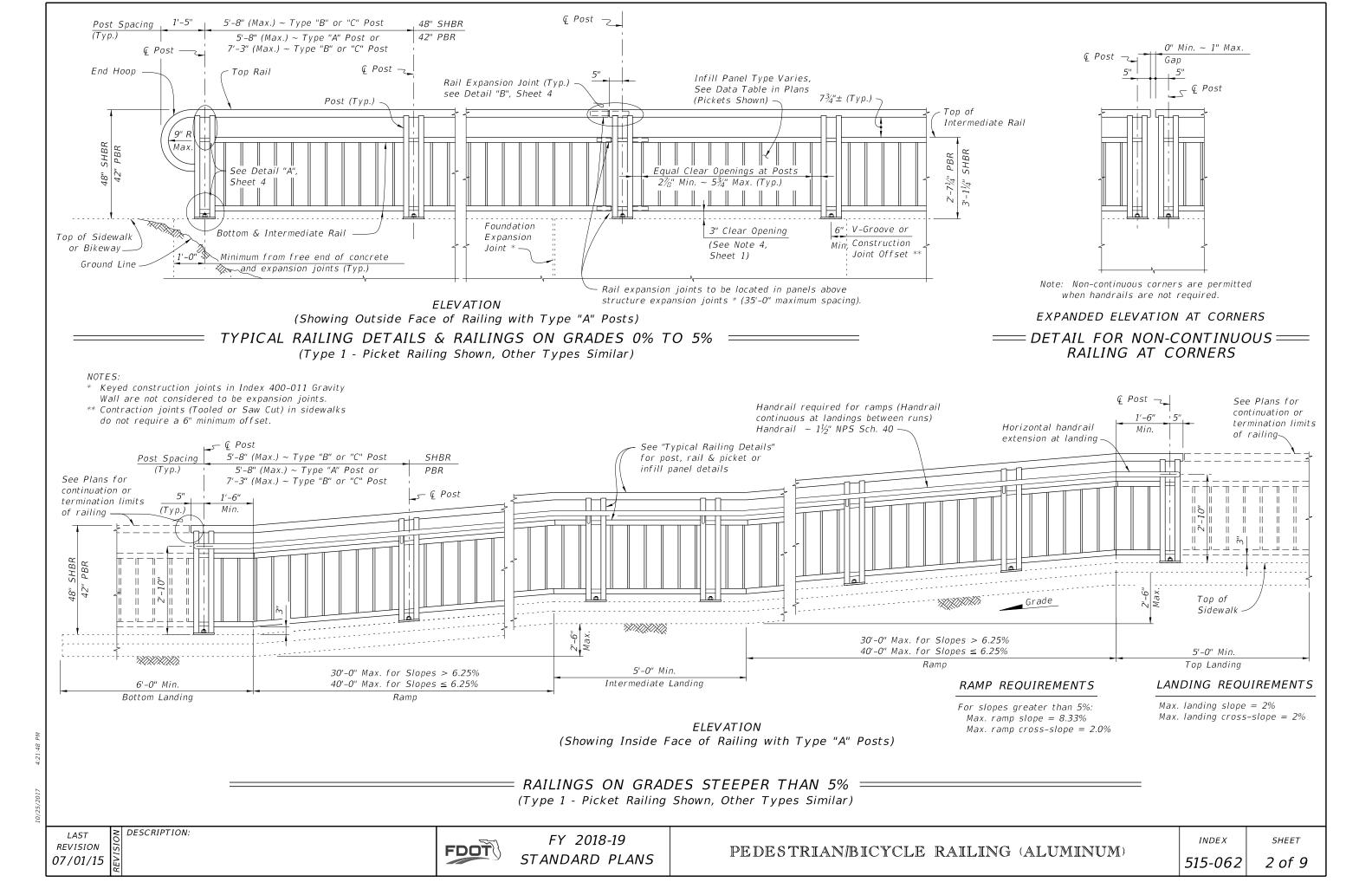
FY 2018-19 STANDARD PLANS

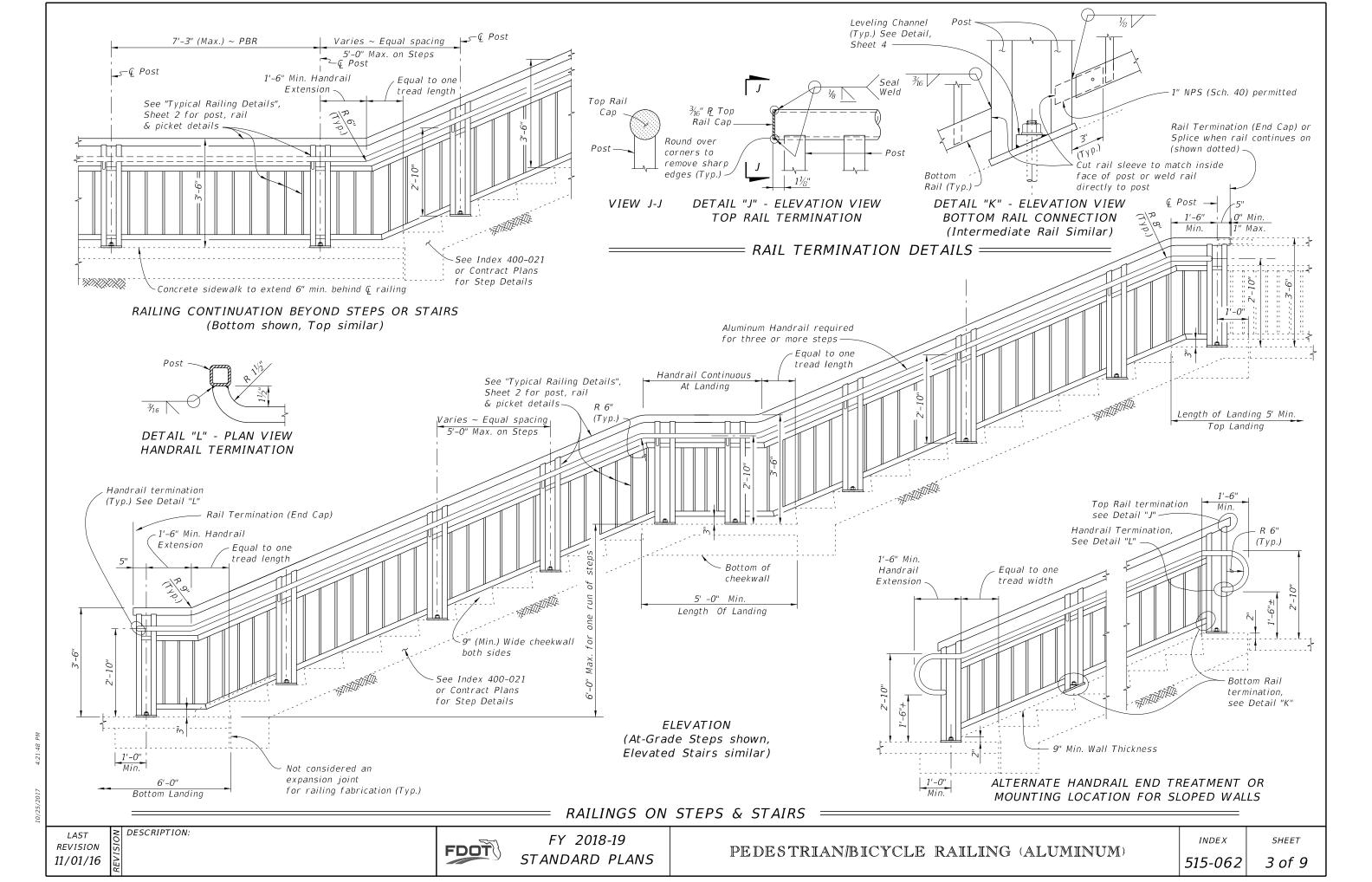
PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

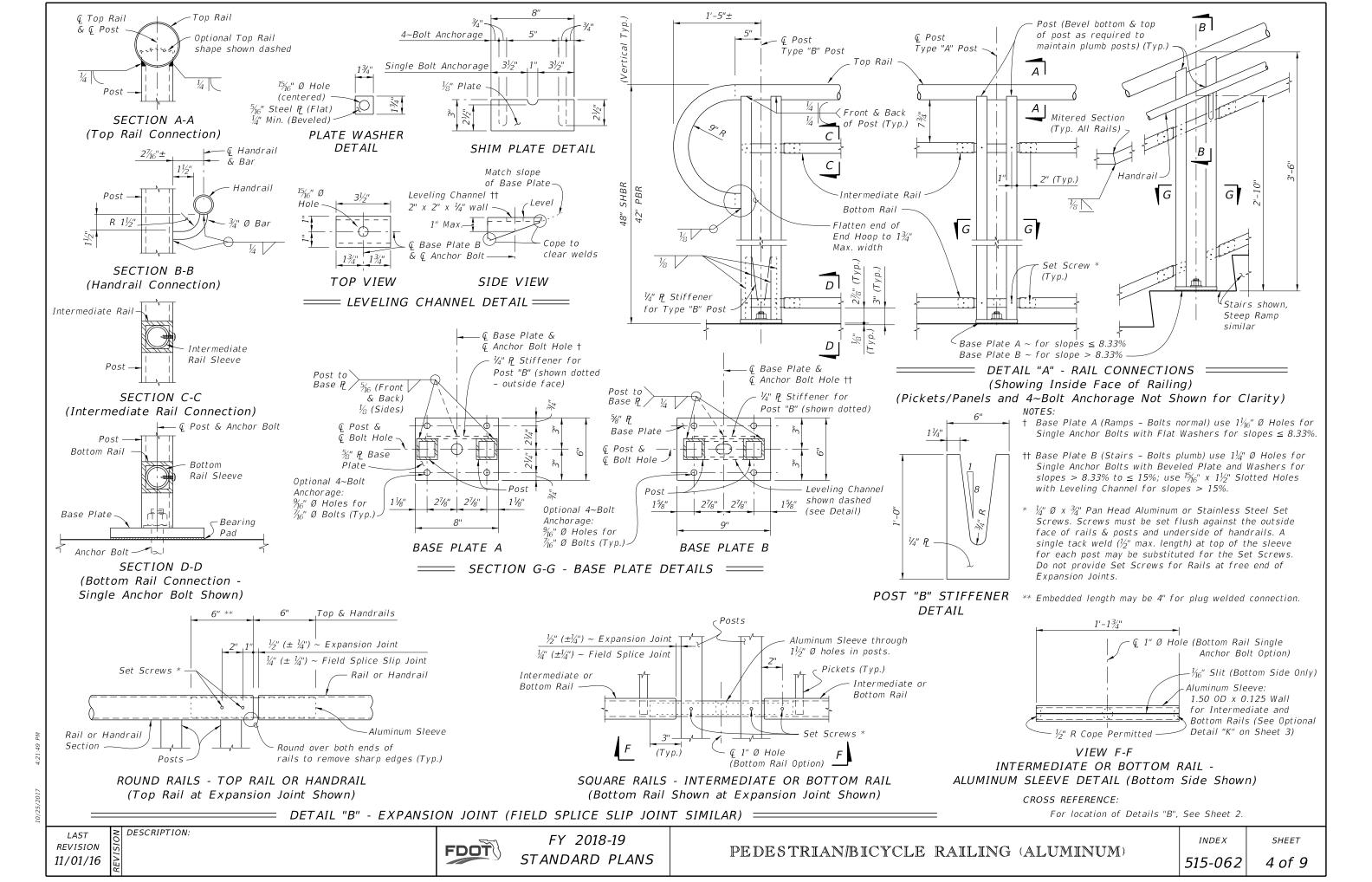
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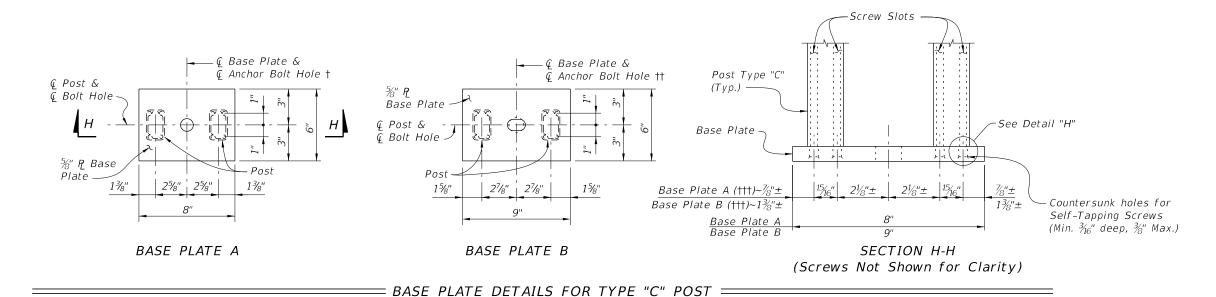
SHEET

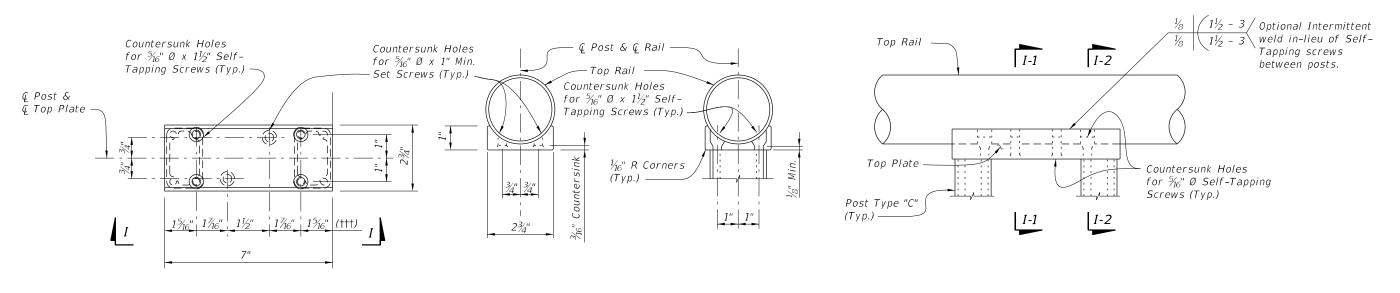
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SECTION "I-2"

= TOP PLATE DETAILS FOR TYPE "C" POST = (Screws Not Shown For Clarity)

- See Sheet 4 for Notes.
- See Sheet 4 for Notes.
- Length varies for beveled posts on grades. Holes must be drilled plumb to align with screw slot.

PLAN

DESCRIPTION: REVISION 11/01/16

FDOT

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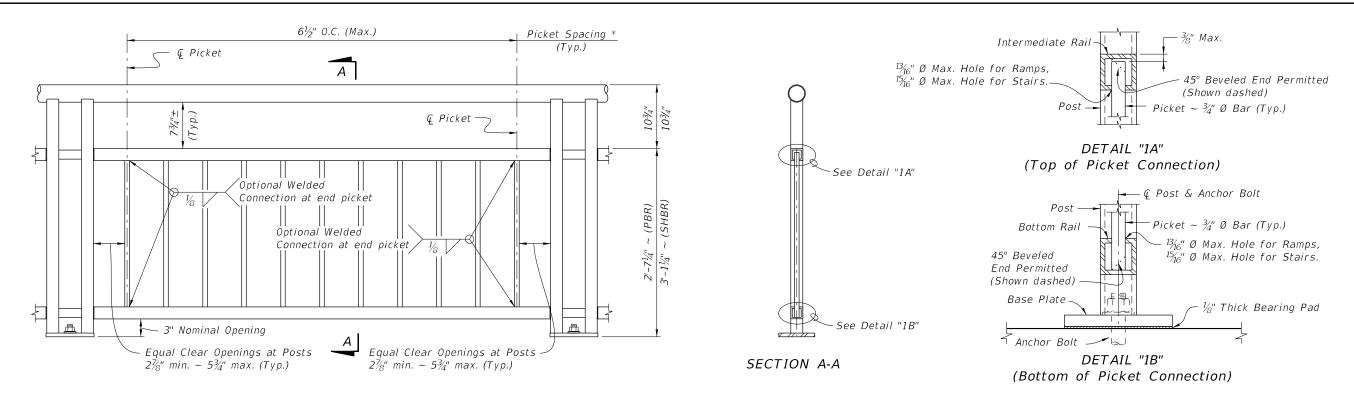
SECTION "I-1"

PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

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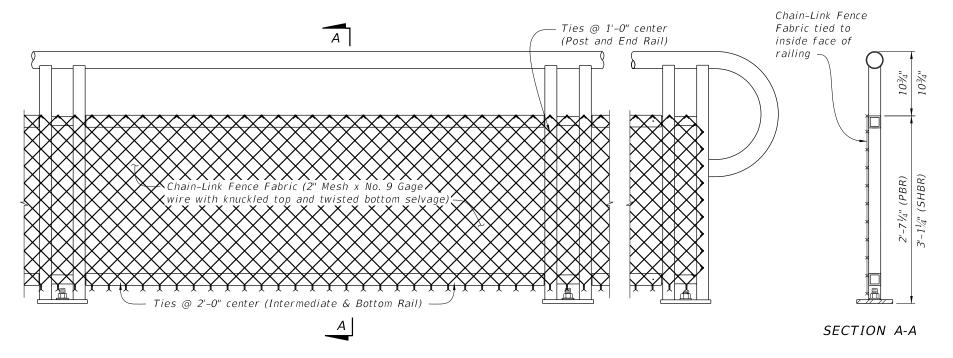
VIEW "I"



TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. See Note 4 (Sheet 1).



TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES:

DESCRIPTION:

1. See Plans for Infill Panel option required.

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS					
COMPONENT	ASTM	COMPONENT INFORMATION			
Chain-Link Fence Fabric (2" mesh with	A392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating			
twisted bottom and knuckled top selvage)	A491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)			
, , , , , , , , , , , , , , , , , , ,	F668	Polyvinyl Chloride (PVC) Coated Steel - No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.			
Tie Wires	F626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.			
Tension Bars	F626	$\frac{3}{16}$ " (min. thickness) x $\frac{3}{4}$ " (min. width) x 2'-3' (min. height) Steel Bars			
Miscellaneous Fence Components	F626	Zinc-Coated Steel			

CHAIN-LINK PANEL NOTE:

Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

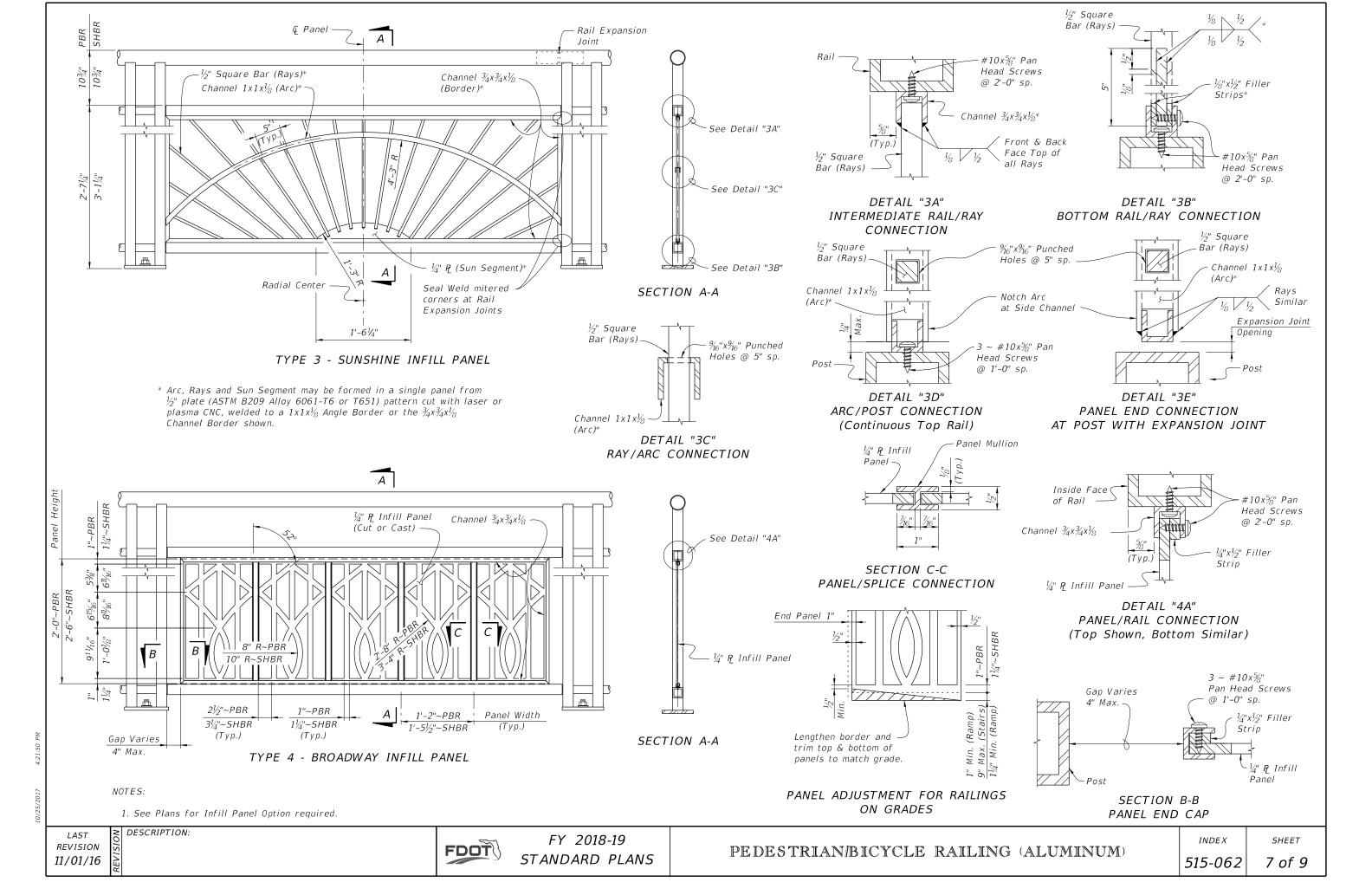
REVISION 11/01/16

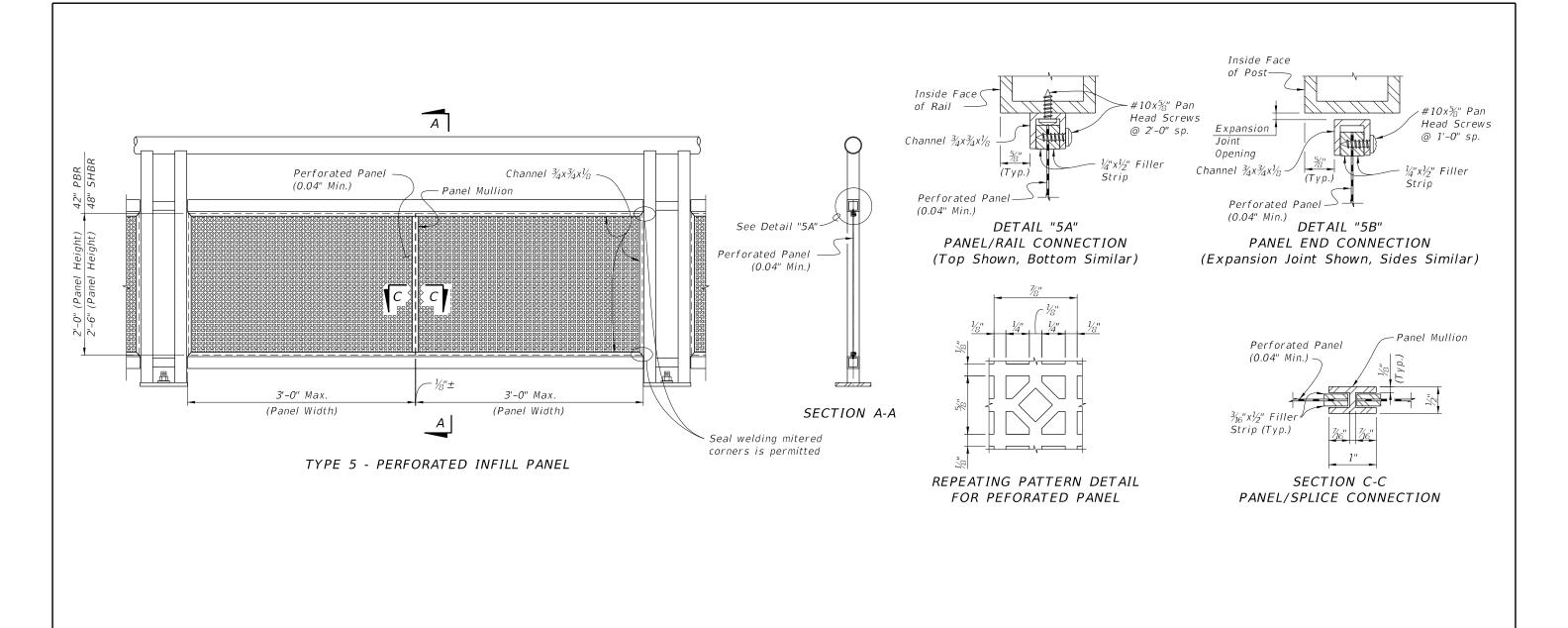
FDOT

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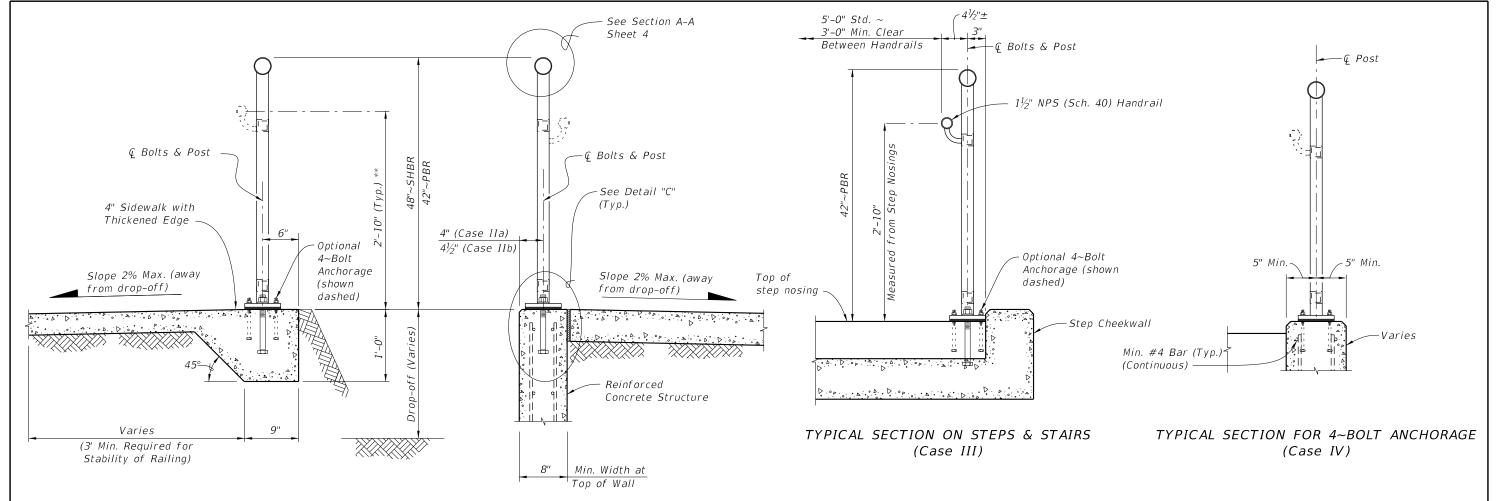




REVISION 11/01/16

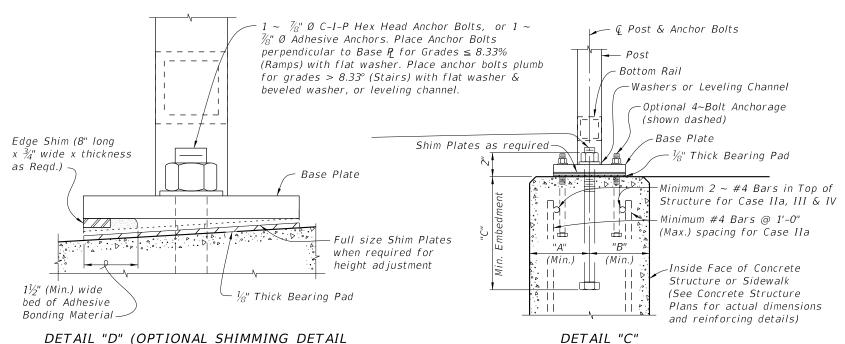
DESCRIPTION:

FDOT



TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

TYPICAL SECTION ON RETAINING WALL (Case II)



	ANCHOR BOLT TABLE						
	CTRUCTURE	DIMENSIONS			ANCHOR LENGTH		44464465
CASE	STRUCTURE TYPE	"A" Edge Dist.	"B" Edge Dist.	"C" Embedment	C.I.P Hex Head Bolt	Adhesive Anchor	ANCHOR SIZE
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	½" Ø
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø
IIb	Gravity Wall Index 400-011	4 ¹ / ₂ "	3½" @ top	1'-0" *	1'-1½"	1'-2"	%" Ø
III	Step Cheekwall	4½"	4½"	9"	10½"	11"	%" Ø
IV	Varies	5"	5"	5"	6½"	7"	7∕16" Ø

^{*} Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".

FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates) (Cast-In-Place Anchor Bolts shown, Adhesive Anchors similar)

DESCRIPTION: **REVISION** 11/01/16

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PEDESTRIAN/BICYCLE RAILING (ALUMINUM)

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^{**} When required; measured from top of sidewalk (Typ.)

NOTES:

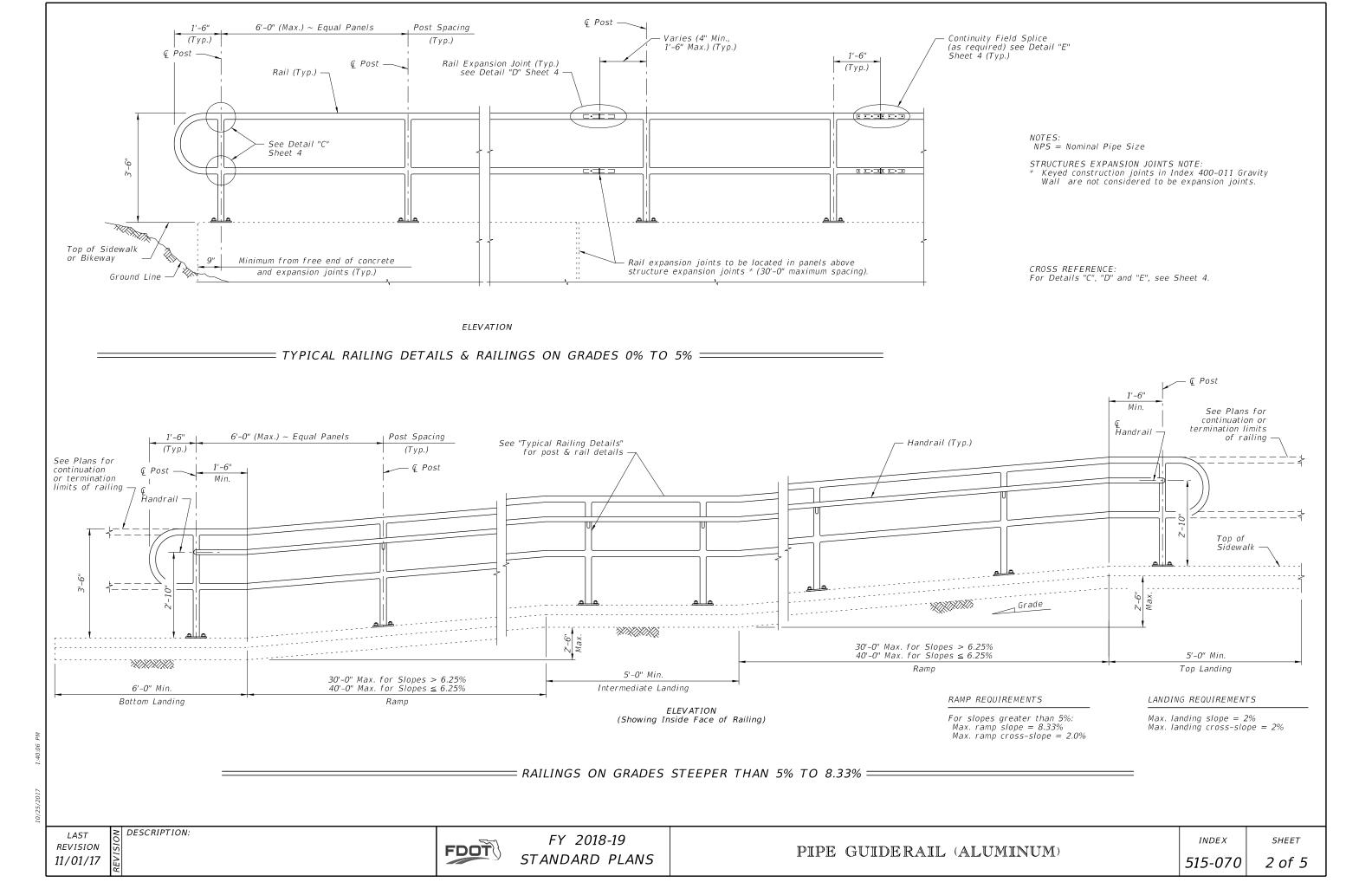
- 1. Shop Drawings are required.
- 2. Work with Specification Section 515.
- A. Pan Head Set Screws: Aluminum Alloy 2024-74 or 7075-T73 or Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM B209, Alloy 6061-T6
- C. Structural Pipe Tube and Bars: ASTM B221 or ASTM B429, Alloy 6061-T6
- D. End Rails 90° bends and corner bends with a maximum 4 foot spacing: Alloy 6063-T6 is permitted.

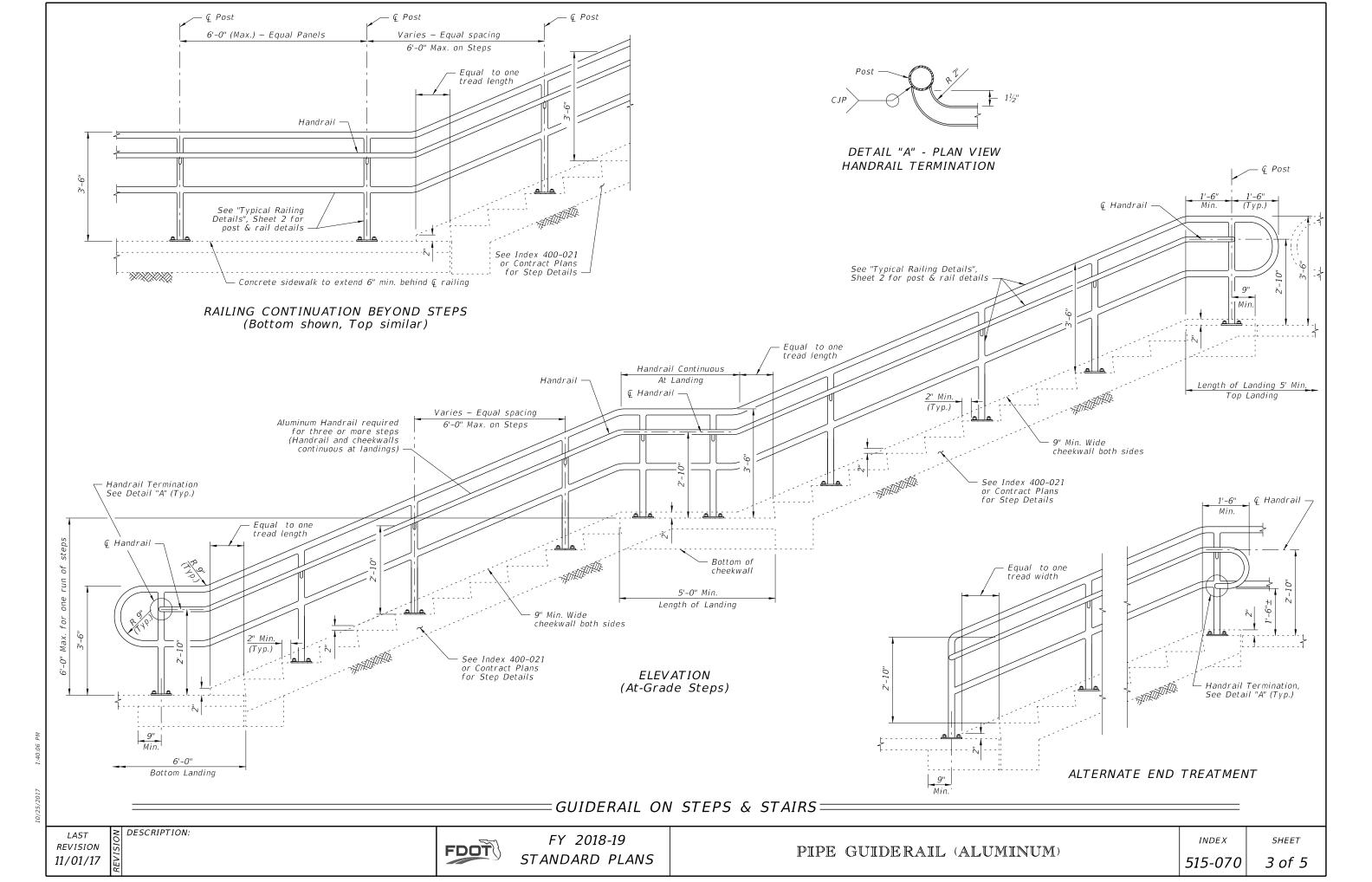
RAILING MEMBER DIMENSIONS TABLE						
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS			
Posts	2" NPS (Sch. 40)	2.375"	0.154"			
Rails	2" NPS (Sch. 40)	2.375"	0.154"			
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) 1.50 ODx0.125 Wall	1.315" 1.500"	0.133" 0.125"			
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A			

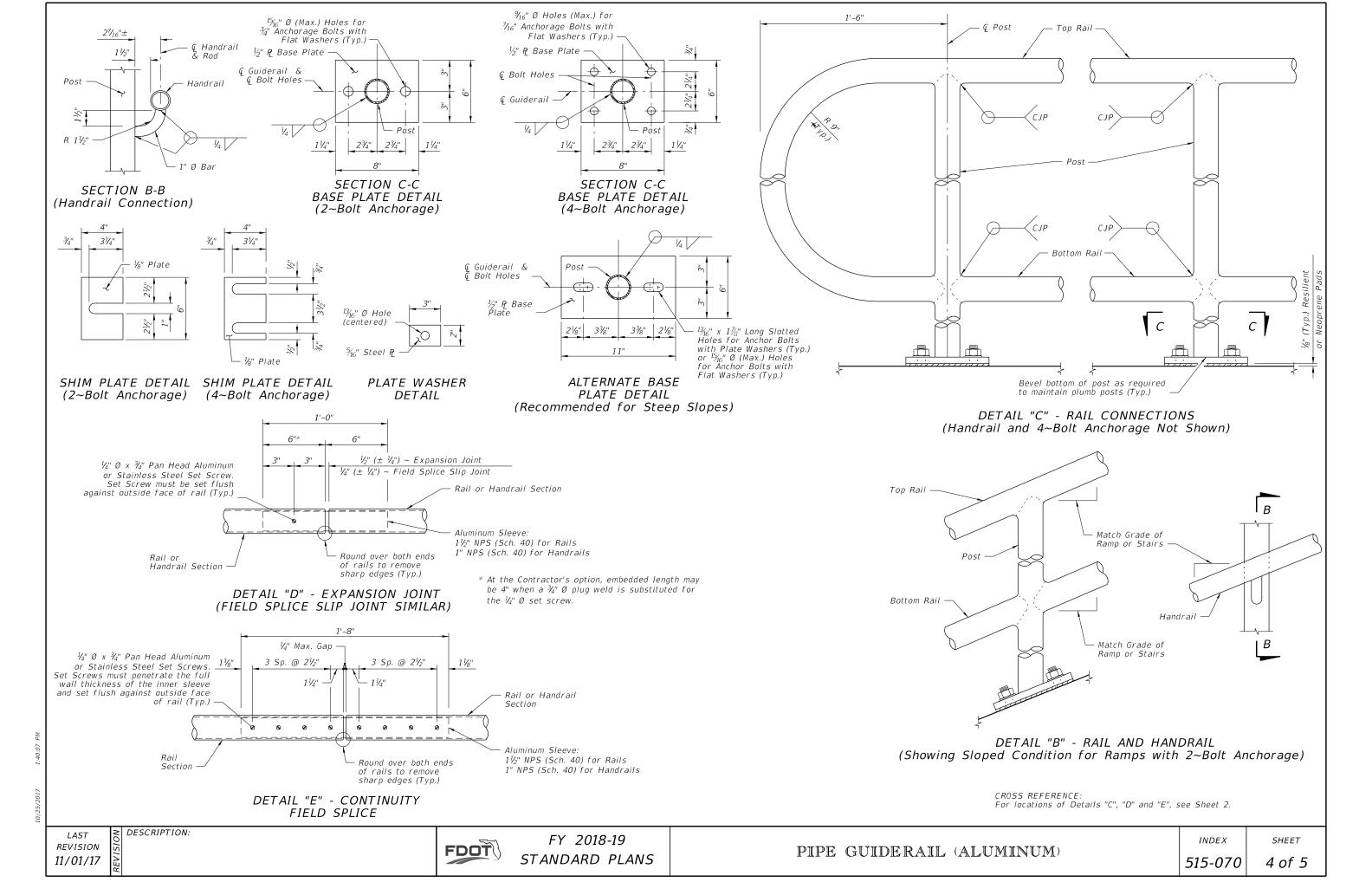
- E. Galvanized Steel Fasteners:
- a. Hex Head Bolts: ASTM A 307 Type 1 or ASTM F1554 Grade 36
- b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods
- c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- F. Aluminum Shims: ASTM B209, Alloy 6061
- G. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specification Sections 515 & 962 for Ancillary Structures.
- 4. Fabrication:
 - A. Place expansion joints at a maximum of 30'-0"spacing
- A. Place expansion joints at a maximum of 30-0 spacing
 B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts.
 C. Continuity field splice (Detail "E"); only use to make the railing continuous for unforeseen field adjustments
 D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9" bend radius or terminated at adjoining sections with a standard end hoop when handrails are not required.
 E. For curved longitudinal alignments, shop bend top and bottom rails and handrails to match the alignment radius.
 E. For changes in tangential longitudinal alignment greater than 45° position posts a maximum of 2"-0" each side.

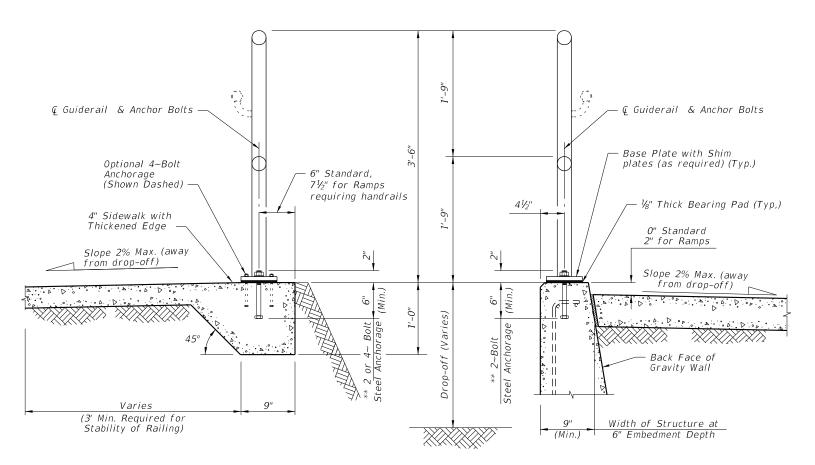
- F. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner, not at the corner apex.
- 5. Handrails are required and must be continuous at landings for:
 - A. Grades Steeper than 5%
- B. Three or more steps
- 6. Cutting of reinforcing steel is permitted for post installed anchor bolts.

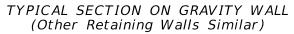
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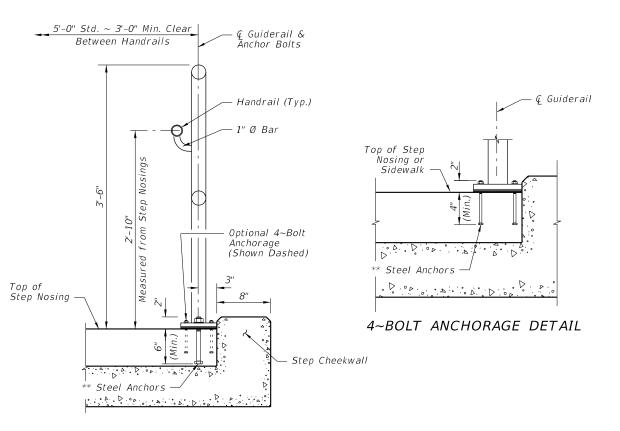




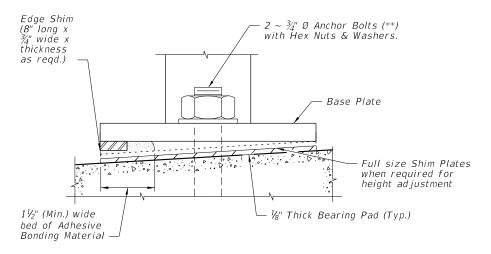






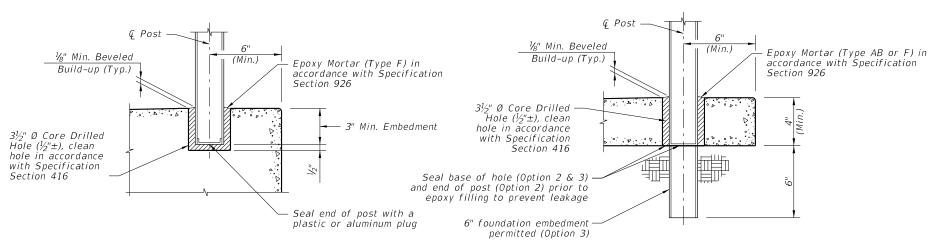


TYPICAL SECTION ON STEPS & STAIRS



DETAIL "F" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

TYPICAL SECTION ON CONCRETE SIDEWALK



SIDEWALK ANCHORAGE DETAIL OPTION 1

SIDEWALK ANCHORAGE DETAIL OPTION 2 & 3

NOTES:

** $2 \sim \frac{3}{4}$ " Ø x 8" or $4 \sim \frac{7}{16}$ " Ø x 6" Steel Anchors:

Galvanized Steel Bolts (As Shown) (C-I-P); Galvanized U-Bolts

Permitted (C-I-P); Galvanized Adhesive Anchors Permitted

*** The minimum embedment for Adhesive Anchors is 6" for 2~Bolt Anchorage or 4" for 4~Bolt Anchorage.

REVISION 11/01/17

DESCRIPTION:

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PIPE GUIDERAIL (ALUMINUM)

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NOTES:

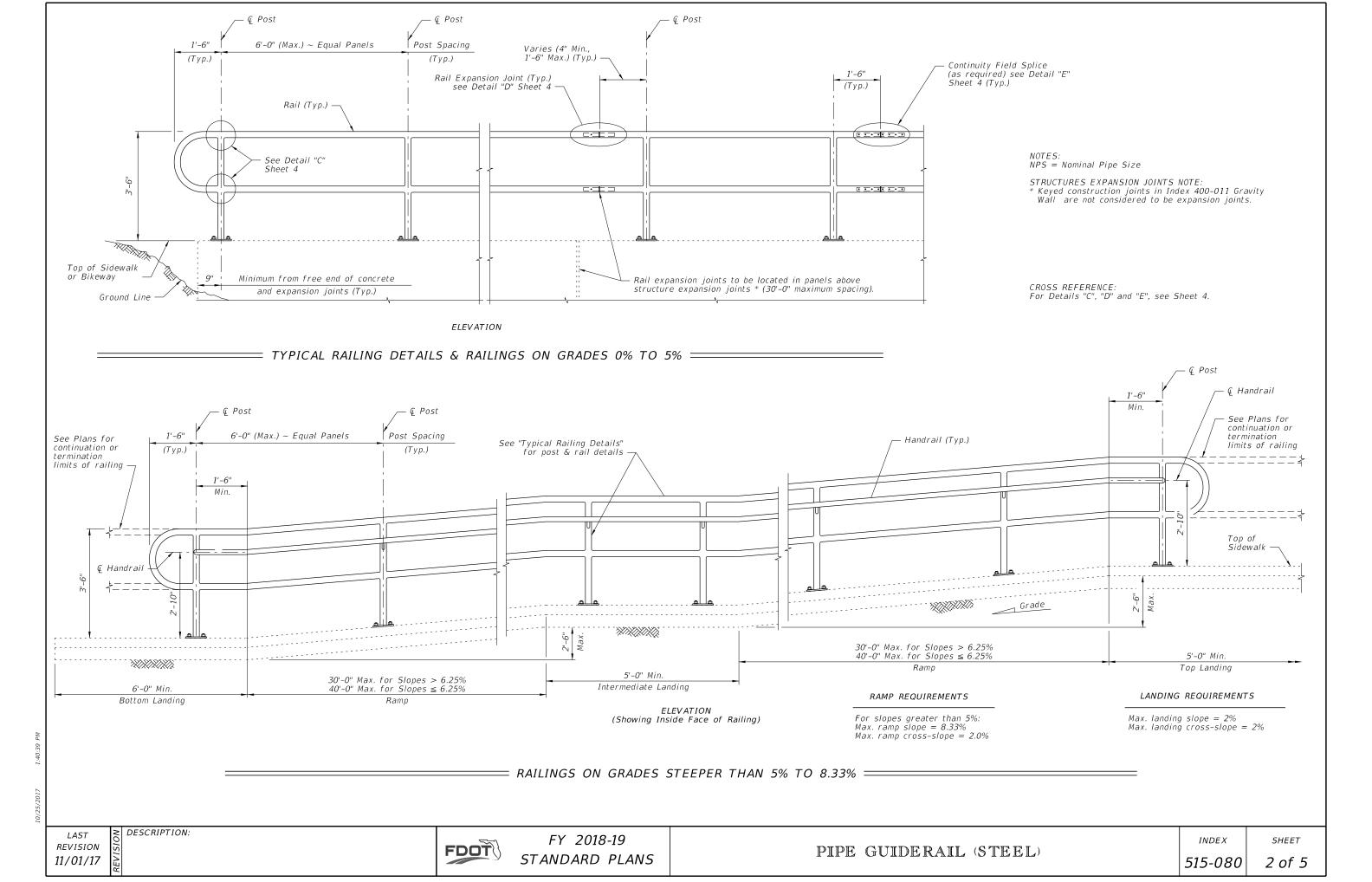
- 1. Shop Drawings are required, refer to Specification Section 515.
- 2. Materials:
- A. Pan Head Set Screws: Stainless Steel (SS) Type 316 or 18-8 Alloy.
- B. Base Plates and Cap Plates: ASTM A36 or ASTM A709 Grade 36
- C. Pipe Rails and Posts: ASTM A53 Grade B for standard weight pipe and ASTM A500 Grade B, C or D or ASTM A501 for Structural Tube.

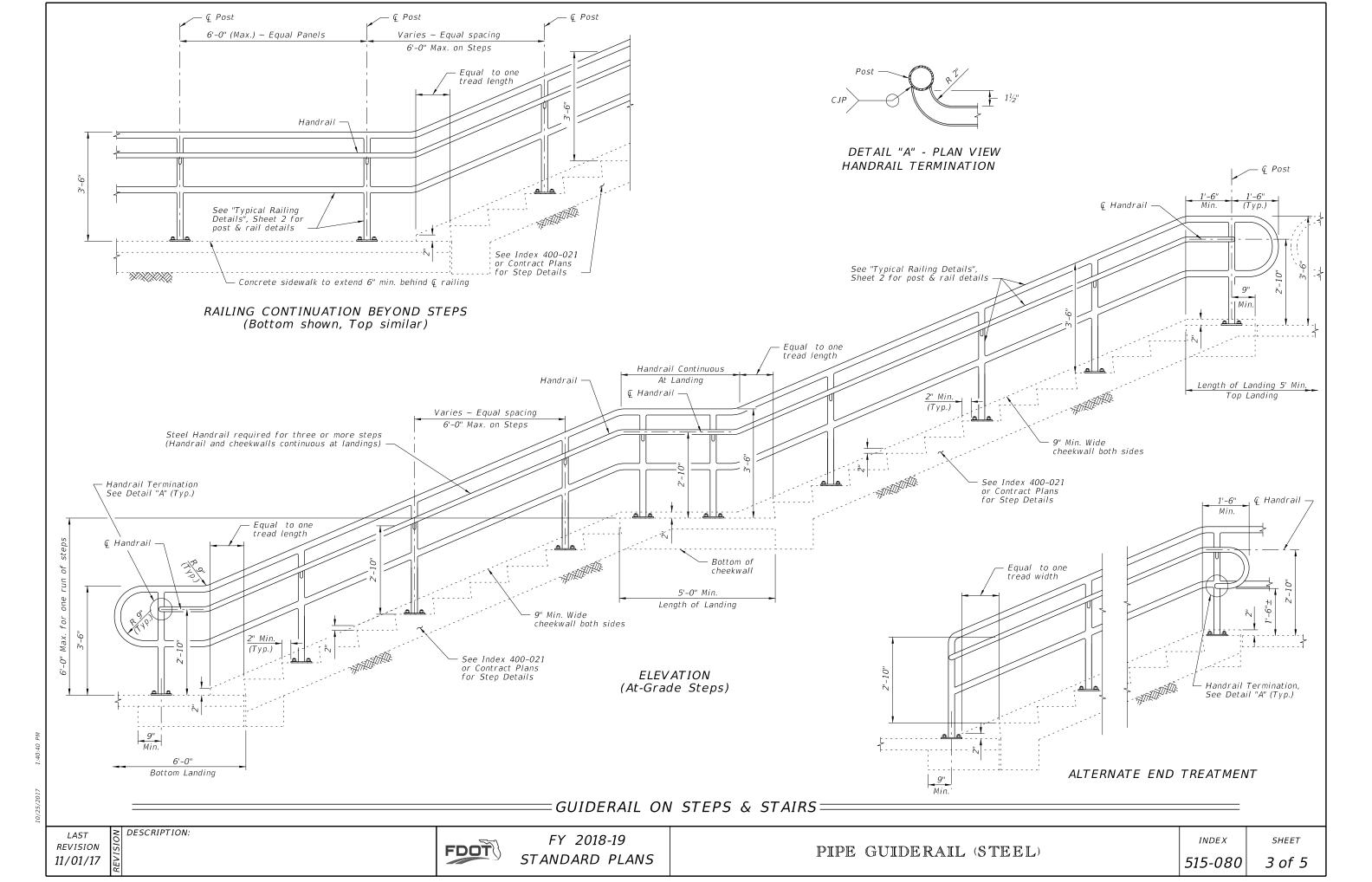
Handrail Support Bars: ASTM A36

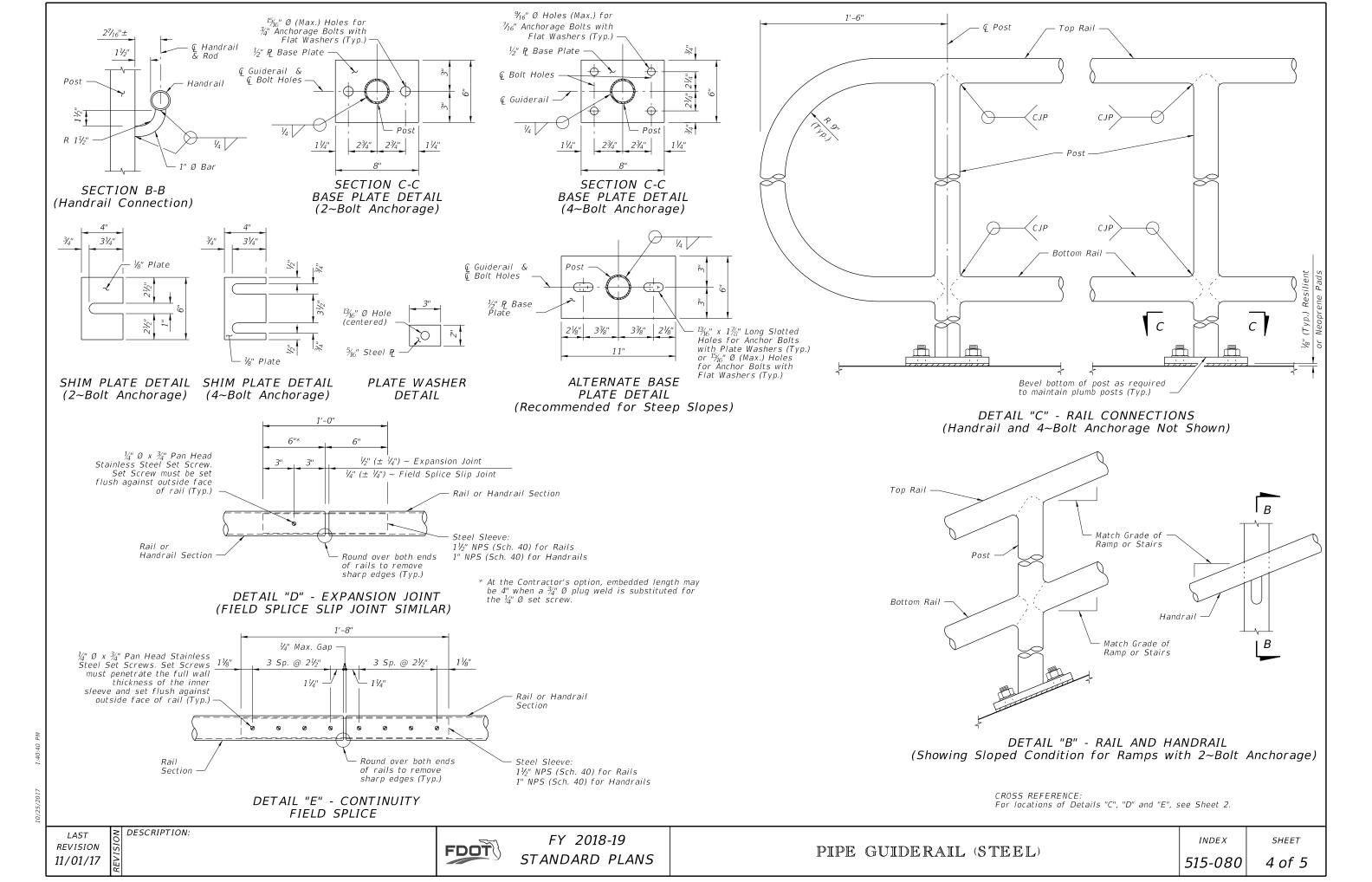
RAILING MEMBER DIMENSIONS TABLE						
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS			
Posts	2" NPS (Sch. 40)	2.375"	0.154"			
Rails	2" NPS (Sch. 40)	2.375"	0.154"			
Rail Joint/Splice Sleeves	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrails Joint/Splice Sleeves	1" NPS (Sch. 40) HSS1.500x0.125	1.315" 1.500"	0.133" 0.125"			
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrail Support Bar	1" Ø Round Bar	1.000"	N/A			

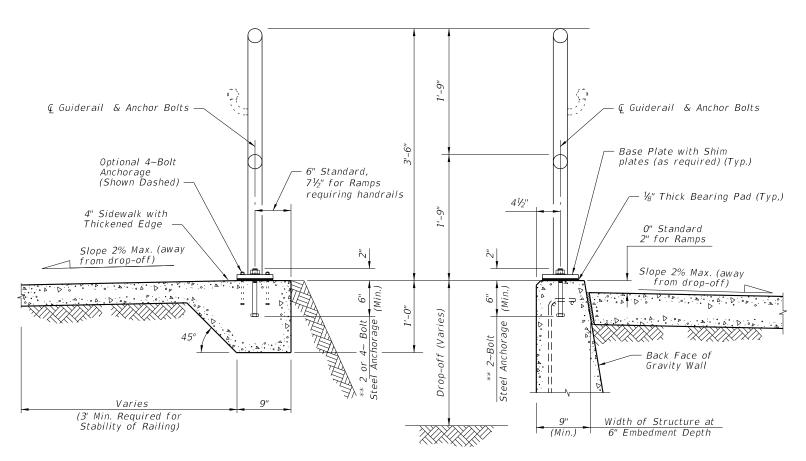
- D. Galvanized Steel Fasteners:
- a. Hex Head Bolts: ASTM A307 Type 1 or ASTM F1554 Grade 36 b. Adhesive Anchors: ASTM F1554 Grade 36 fully threaded rods
- c. Hex Nuts: ASTM A563
- d. Flat Washers: ASTM F436
- E. Aluminum Shims: ASTM B209, Alloy 6061
- F. Bearing Pads: Plain, Fabric Reinforced, or Fabric Laminated meeting requirements of Specification Sections 515 and 962 for Ancillary Structures.
- 3. Fabrication:
- A. Place expansion joints at a maximum of 30'-0"spacing.
- B. Field splices are similar to the expansion joint detail and may be approved by the Engineer to facilitate handling; but top rail must be continuous across a minimum of two posts.
- C. Continuity field splice (Detail "E") only use to make the railing continuous for unforeseen field adjustments
- D. Corners and changes in tangential longitudinal alignment may be made continuous with a 9"bend radius or terminated at adjoining sections with a standard end hoop when handrails are not required.
- E. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- F. For changes in tangential longitudinal alignment greater than 45°, positioned posts a maximum of 2'-0" each side of the corner, not at the corner apex.
- 4. Handrails are required and must be continuous at landings for:
- A. Grades Steeper than 5%,
- B. Three or more steps
- 5. Cutting of reinforcing steel is permitted for adhesive anchor bolt installations.

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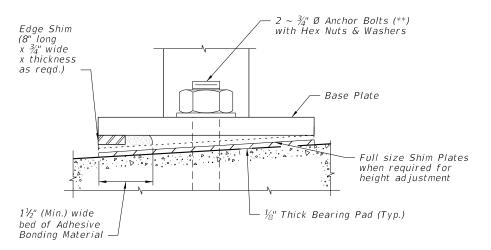






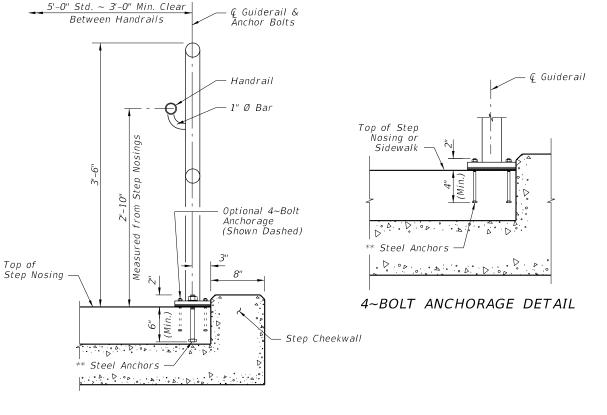




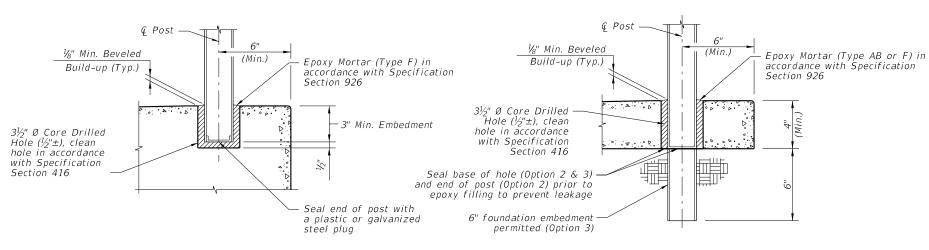


DETAIL "F" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

TYPICAL SECTION ON GRAVITY WALL (Other Retaining Walls Similar)



TYPICAL SECTION ON STEPS & STAIRS



OPTIONAL SIDEWALK ANCHORAGE DETAIL

SIDEWALK ANCHORAGE DETAIL OPTION 2 & 3

2 $\sim \frac{3}{4}$ " Ø x 8" or 4 $\sim \frac{7}{16}$ " Ø x 6" Steel Anchors: Galvanized Steel Bolts (As Shown) (C-I-P); Galvanized U-Bolts Permitted (C-I-P); Galvanized Adhesive Anchors Permitted (*); Expansion Anchors Not Permitted.

*** The minimum embedment for adhesive anchors is 6" for 2~Bolt Anchorage or 4" for 4~Bolt Anchorage.

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DESCRIPTION:

FDOT

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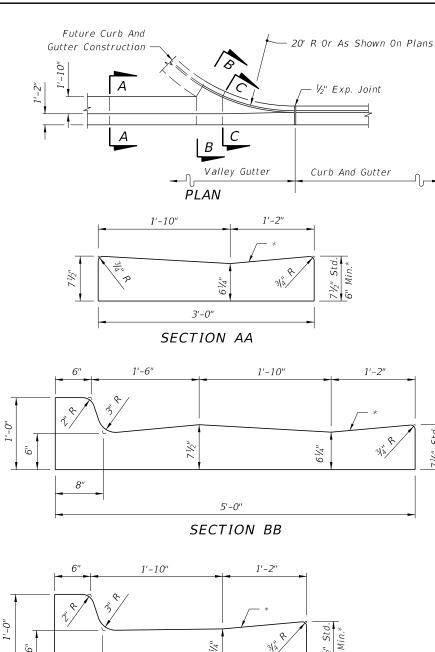
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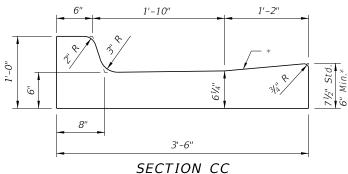
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PIPE GUIDERAIL (STEEL)

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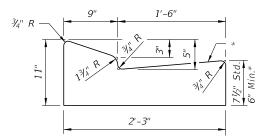




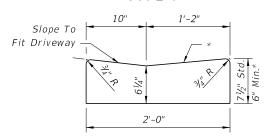
VALLEY GUTTER

- * When used on high side of roadways, the cross slope of the gutter shall match the cross slope of the adjacent pavement. The thickness of the lip shall be 6", unless otherwise shown on plans.
- ☑ Rotate entire section so that gutter cross slope matches slope of adjacent circulating roadway pavement.

For use adjacent to concrete or flexible pavement. For details depicting usage adjacent to flexible pavement, see Sheet 2. Expansion joint, preformed joint filler and joint seal are required between curb & gutter and concrete pavement only, see Sheet 2.

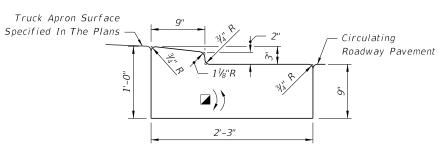


TYPE E TYPE F

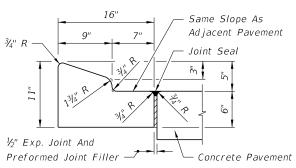


Note: To be paid for as parent curb.

DROP CURB - Standard Shoulder Line Shoulder Pavement Earth Berm 3'-6" SHOULDER GUTTER

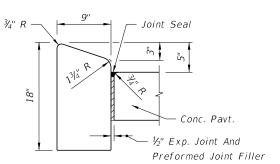


TRAFFIC BEARING SECTION FOR USE IN ROUNDABOUT CENTRAL ISLAND CONSTRUCTION TYPE RA

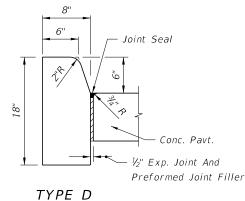


For details depicting usage adjacent to flexible pavement, see Sheet 2.

TYPE A



TYPE B



Note: For use adjacent to concrete or flexible pavement, concrete shown. Expansion joint, preformed joint filler and joint seal are required between curbs and concrete pavement only, see Sheet 2.

CONCRETE CURB

CONCRETE CURB AND GUTTER

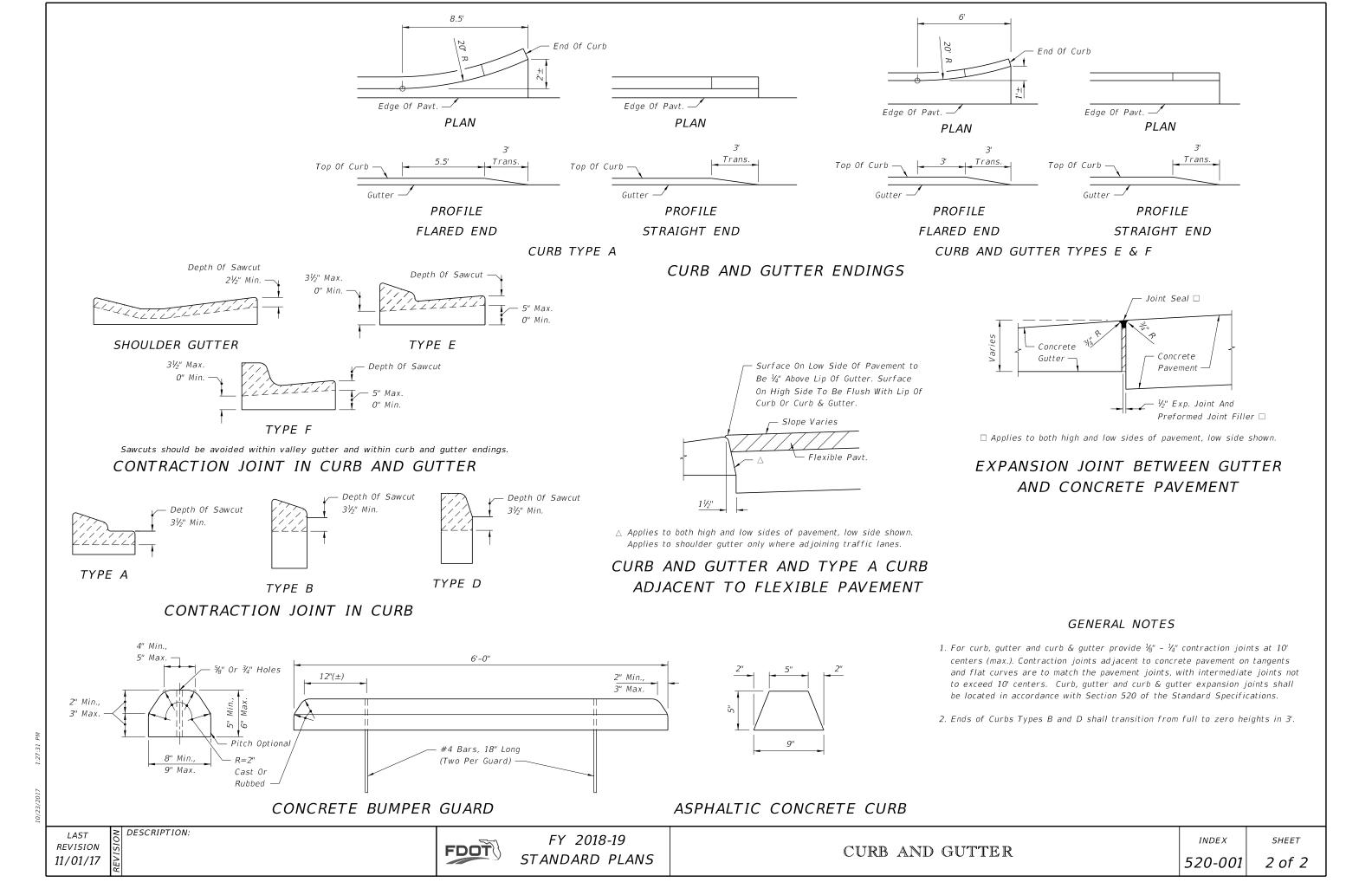
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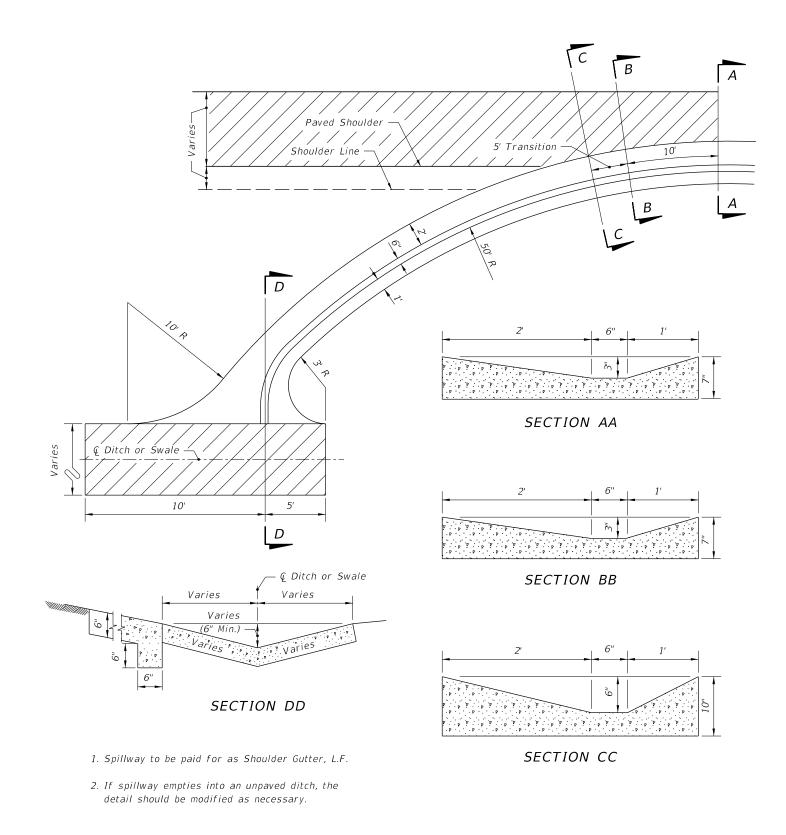
DESCRIPTION:

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DETAIL OF CONCRETE SPILLWAY AT END OF SHOULDER GUTTER (TO BE USED WHERE INLETS, PIPES & ENDWALLS ARE IMPRACTICAL)

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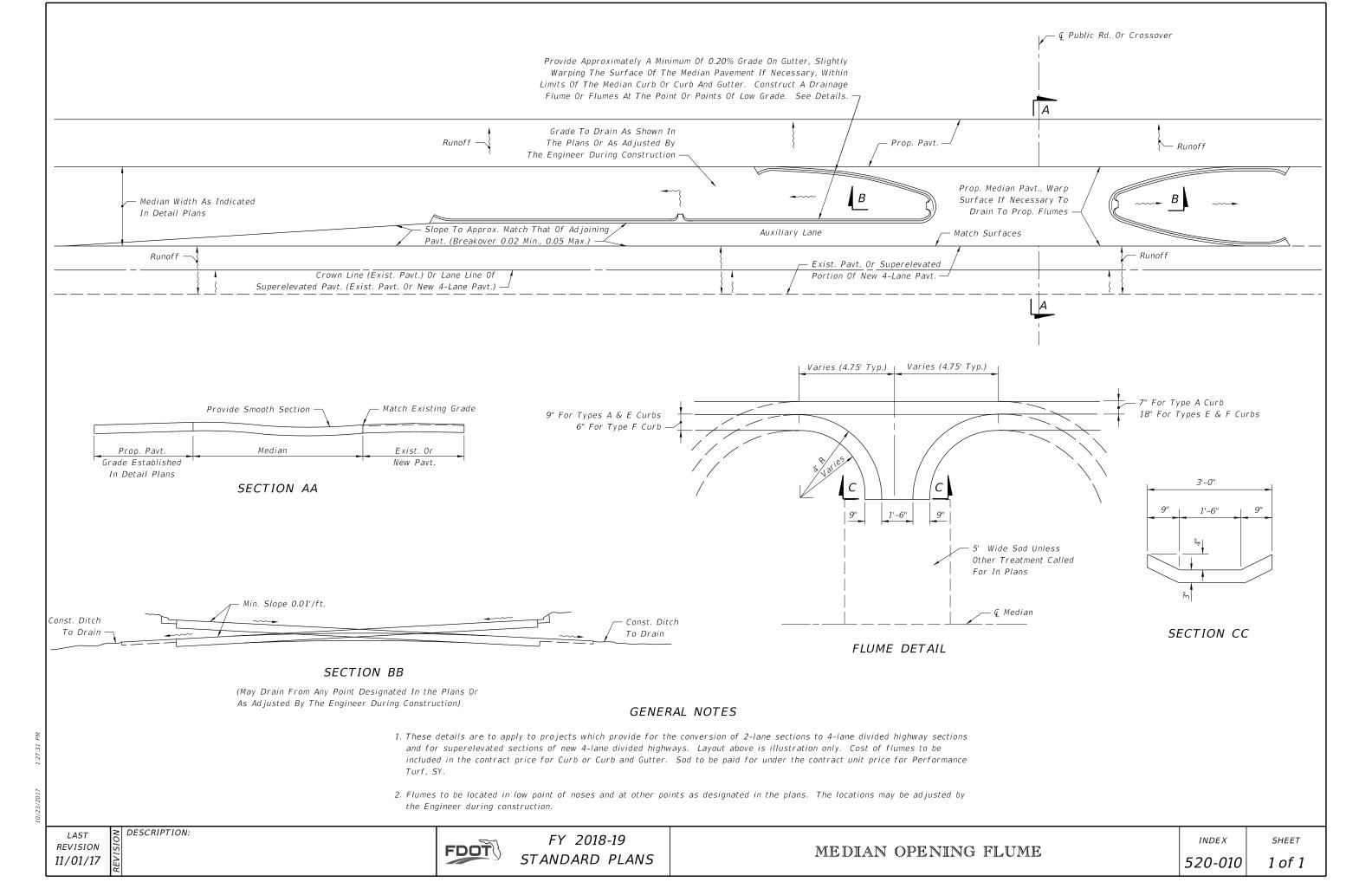
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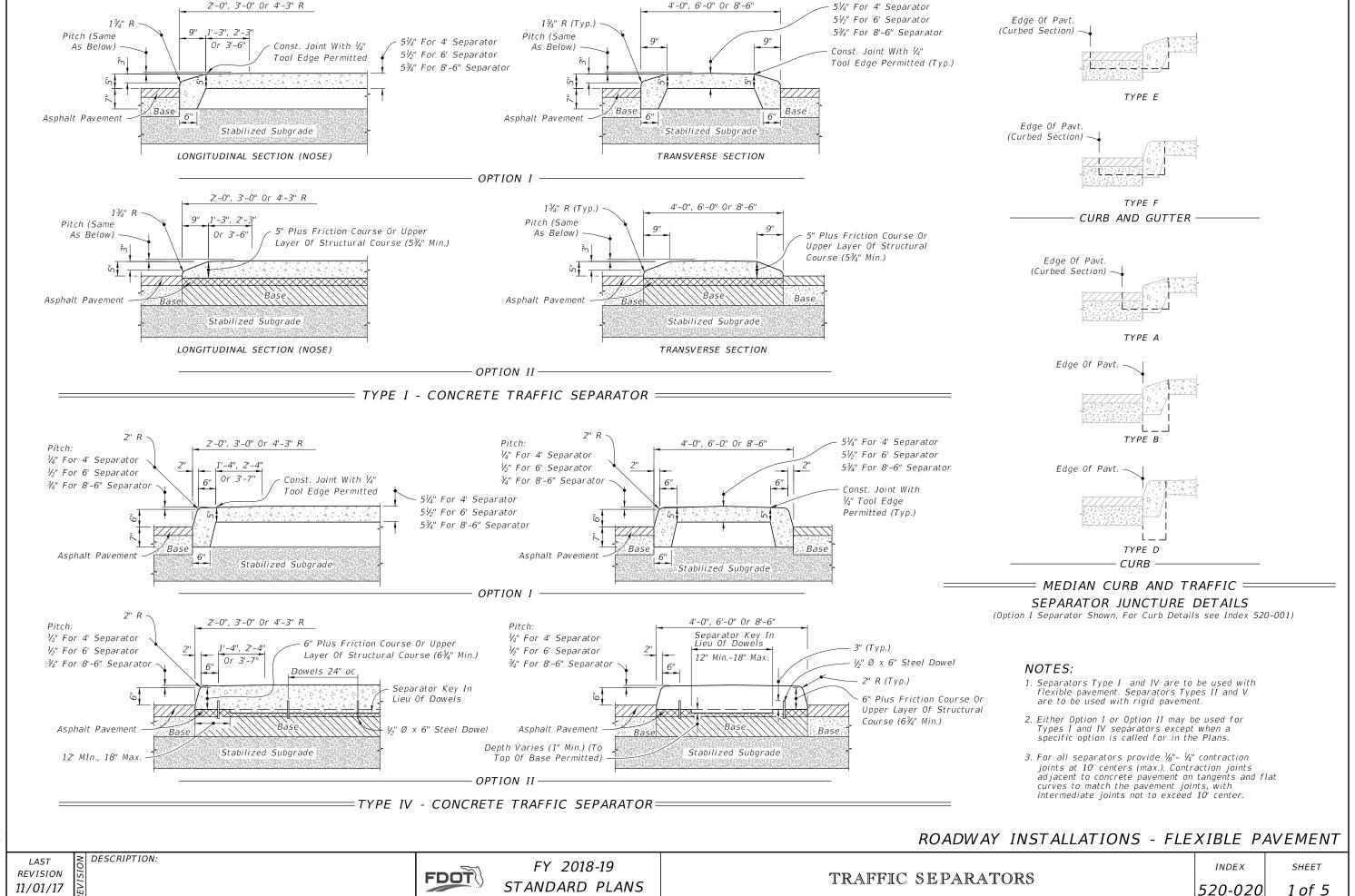
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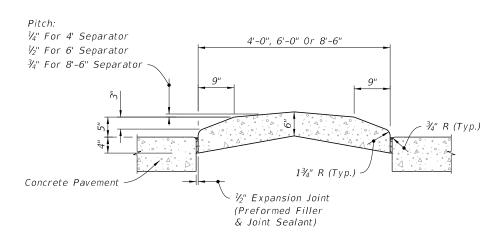




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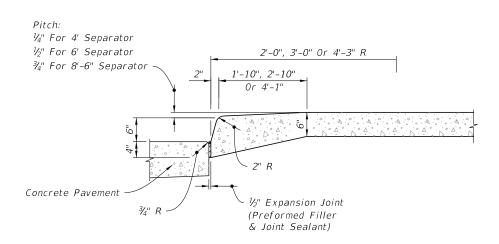
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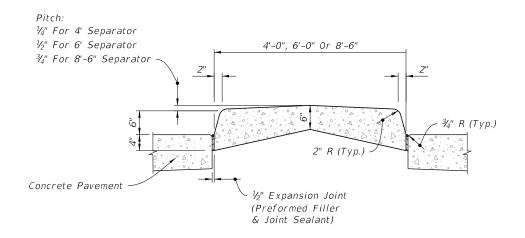


LONGITUDINAL SECTION (NOSE)

TRANSVERSE SECTION

= TYPE II - CONCRETE TRAFFIC SEPARATOR =





LONGITUDINAL SECTION (NOSE)

TRANSVERSE SECTION

= TYPE V - CONCRETE TRAFFIC SEPARATOR =

ROADWAY INSTALLATIONS - RIGID PAVEMENT

REVISION 11/01/17

FDOT

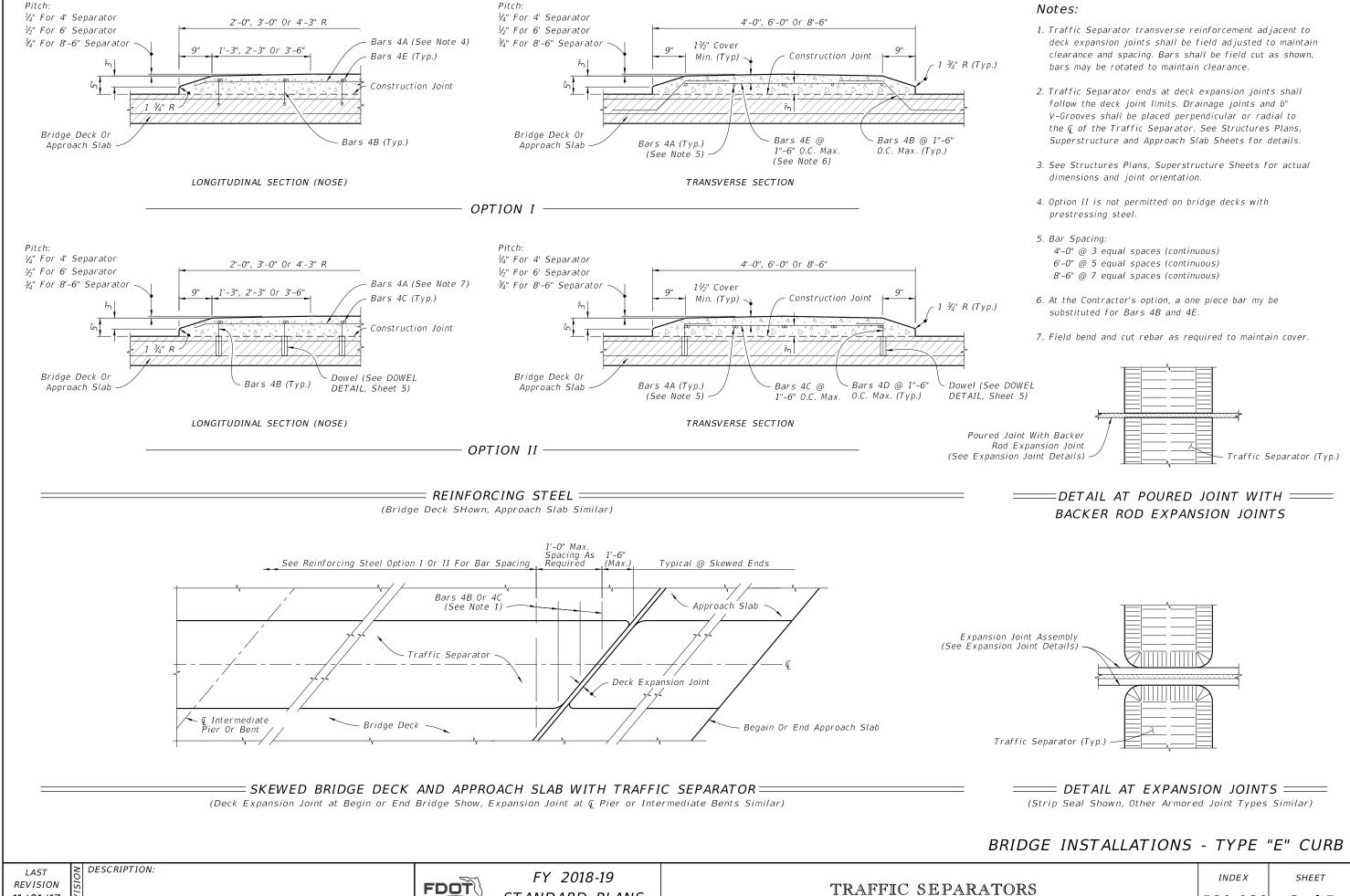
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TRAFFIC SEPARATORS

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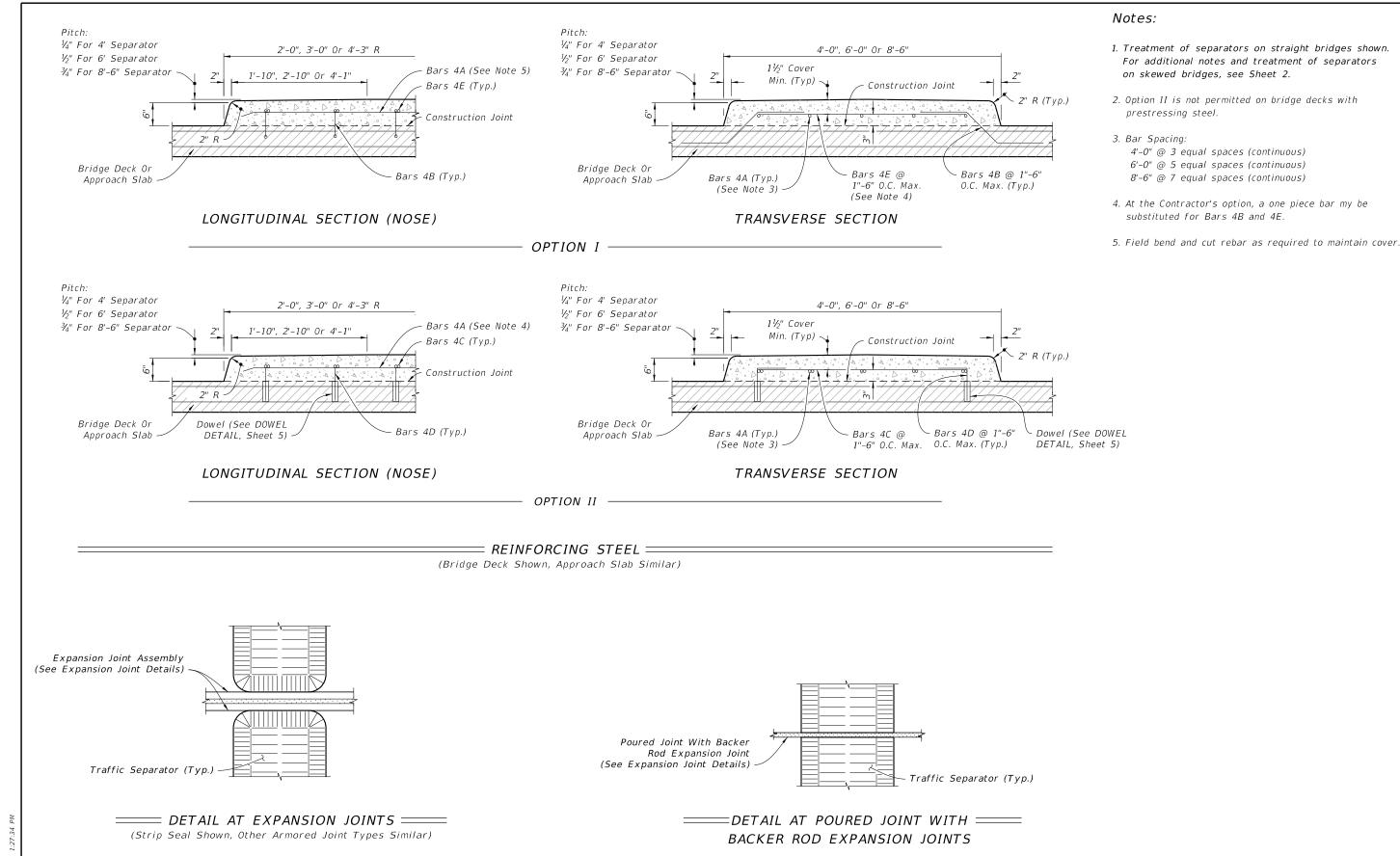


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BRIDGE INSTALLATIONS - TYPE "F" CURB

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DESCRIPTION:

FDOT

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TRAFFIC SEPARATORS

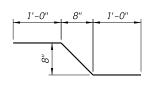
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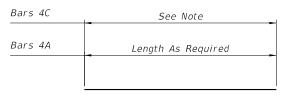


Bars 4A & 4E

Bar 4B

Length of Bars 4E is 2'-5" for 4'-0" Separator. Length of Bars 4E is 4'-5" for 6'-0" Separator. Length of Bars 4E is 6'-11" for 8'-6" Separator.

— OPTION I —





Bars 4A & 4C

Bar 4D

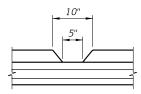
Length of Bars 4C is $2'-4\frac{1}{2}"$ for 4'-0" Separator. Length of Bars 4C is $4'-4\frac{1}{2}''$ for 6'-0'' Separator. Length of Bars 4C is $6'-10\frac{1}{2}''$ for 8'-6'' Separator.

— OPTION II —

REINFORCING STEEL NOTES:

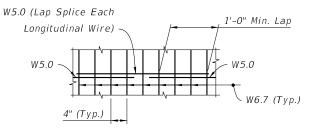
- 1. All dimensions are out to out.
- 2. The 8" vertical dimension shown for Bars 4B and 4D are based on a slab 81/3" thick or greater without a wearing surface. If slab thickness is less than 81/3", decrease this dimension by an amount equal to the difference in thickness. If a wearing surface is to be provided, increase this dimension by an amount equal to the wearing surface thickness.

CONVENTIONAL REINFORCING STEEL BENDNG DIAGRAMS



See Structures Plans, Superstructure Sheets for location(s) of drainage joints. Locations for drainage joints shall be limited to the constant width section of separator.

> = DRAINAGE JOINT DETAIL = (For 5" Opening Or Less)



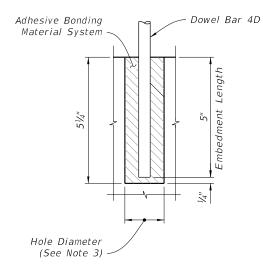
SPLICE DETAIL (Between WWR 3 x 4 - W5.0 x W6.7 Sections)

OPTION A: Use Welded Wire Reinforcement 3 x 4 - W5.0 x W6.7 as required by plans in place of Bars 4A, 4B and 4E. Bend the Welded Wire Reinforcement to the dimensions of Bar 4B shown in the Bending Diagram for Reinforcing Steel Option I.

OPTION B: Use Welded Wire Reinforcement 3 x 4 - W5.0 x W6.7 as required by plans in place of Bars 4A and 4C shown in Reinforcing Steel Option II.

NOTE: Welded Wire Reinforcement to consist of smooth wire meeting the requirements of Specification Section 931.

== ALTERNATE REINFORCING STEEL DETAILS===== (Welded Wire Reinforcement)



DOWEL NOTES:

- 1. Shift Dowel Holes to clear if existing reinforcement is encountered.
- 2. Provide and install an adhesive bonding material system in accordance with Sections 416 and 937 of the Specifications.
- 3. The dowel hole diameter is to meet adhesive bonding material system manufacture's requirements.

DOWEL DETAIL

ESTIMATED TRAFFIC SEPARATOR QUANTITIES:

CONSTANT WIDTH OF SEPARATOR:

<u> TYPE "E"</u>		<u>TYPE "F"</u>		
4'-0"	Width = 0.056 CY per Ft.	- 0.072 CY per Ft		
6'-0"	Width = 0.089 CY per Ft.	- 0.112 CY per Ft		
8'-6"	Width - 0 132 CY ner Et	- 0.164 CY ner Et		

NOSE:

	<i>TYPE "E"</i>		TYPE "F
4'-0"	Width = 0.080 CY	-	0.109 CY
6'-0"	Width = 0.193 CY	-	0.257 CY
8'-6"	Width = 0.403 CY	_	0.536 CY

REINFORCING STEEL:

(All quantities are based on an $8\frac{1}{2}$ " slab.)

OPTION I:

4'-0" Width - 6.37 Lbs. per Ft. 6'-0" Width - 8.60 Lbs. per Ft. 8'-6" Width - 11.05 Lbs. per Ft.

OPTION II:

4'-0" Width - 4.77 Lbs. per Ft. 6'-0" Width - 7.00 Lbs. per Ft. 8'-6" Width - 9.45 Lbs. per Ft.

BRIDGE INSTALLATIONS - TYPE "E" AND "F" CURB

REVISION 11/01/17

DESCRIPTION:

FY 2018-19 STANDARD PLANS

TRAFFIC SEPARATORS

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5	Median Barrier - Grade Separated	
6	Median Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding - Symmetrical	
7	Median Barrier - 56" Height Section for Barrier-Mounted Sign Support Shielding - Asymmetrical	
8	Median Barrier - 38" Height Split Section for Stand-Alone Sign Support Shielding	
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22	Reinforcing Bar Bending Diagrams	

GENERAL NOTES:

- 1. CONCRETE: Use Class II concrete for all barriers constructed in slightly aggressive environments, and use Class IV Concrete for all barriers constructed in moderately or extremely aggressive environments. On all exposed surfaces, apply a Class 3 surface finish in accordance with Specification 400.
- 2. STEEL BAR REINFORCEMENT: Where required to maintain continuity, provide lap splices of at least 18 inches for No. 4 bars and 20 inches for No. 5 bars, unless otherwise shown herein (including shorter splices as provided by the default bar bending diagrams).

The default reinforcing details shown herein, including bar shapes and lap splice positions, are intended to show required steel locations and provide for a constructible design. However, with the approval of the Engineer, alternate steel configurations may be used in the same locations shown herein, given that the equivalent strength reinforcing is provided and the cover, maximum spacing, and continuity requirements are maintained.

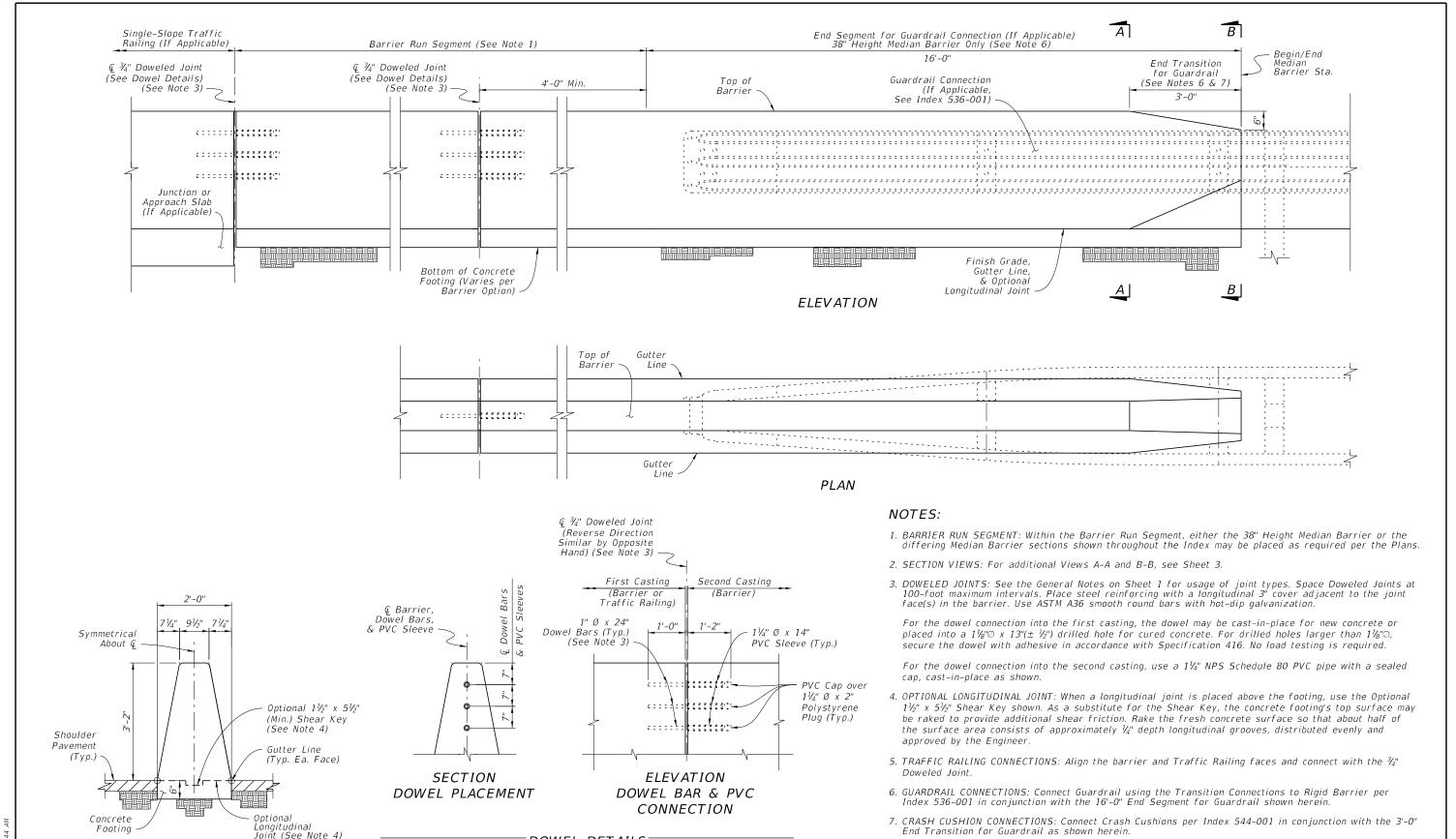
- 3. OPTIONAL WELDED WIRE REINFORCEMENT: With the approval of the Engineer, steel welded wire reinforcement in accordance with Specification 415 may be substituted for the steel bars shown herein. Place the welded wire in the same locations specified for the steel bars, and maintain the equivalent strength, cover, maximum spacing, and continuity requirements.
- 4. TOP FACE LONGITUDINAL REINFORCEMENT: Unless otherwise specified, the longitudinal reinforcement shown closest to the top face of the barrier has a maximum cover of $4\frac{1}{2}$ ", measured from the top face of the barrier.
- 5. MINIMUM BARRIER LENGTH: Unless otherwise shown in the Plans, the minimum Concrete Barrier length is 40 feet.
- 6. CONSTRUCTION JOINTS: Install Construction Joints only as needed for discontinuous concrete casting or cold joints. Maintain continuity of steel reinforcement across Construction Joints. Construction Joints are classified herein as Transverse Joints or Longitudinal Joints.

Transverse Joints are permitted at 20-foot or greater intervals along the barrier. For Tall Grade-Separated Sections, see Sheet 5 for additional Transverse Joint requirements.

Longitudinal Joints are only permitted where indicated in the following details and notes, with a vertical position tolerance of $\pm 1\frac{1}{2}$ " from the locations shown.

- 7. DOWELED JOINTS: As shown in the Dowel Details on Sheets 2 & 12, install ¾" Doweled Joints for Concrete Barrier connections to Pier Protection Barrier and Traffic Railings. Doweled Joints are also required for expansion mitigation in Median Barrier as defined per Sheets 2 & 5. Doweled Joints are not permitted within Grade-Separated Median Barrier.
- 8. CRACK CONTROL V-GROOVES: At 20-foot intervals, place $\frac{3}{8}$ depth V-grooves that run vertically and/or transversely in the front, top, and back faces of barriers. The V-grooves can be either molded or scored while the concrete is still plastic.
- 9. SUBGRADE: Compact the top layer of subgrade with Type B Stabilization, LBR 40 (12 in.).
- 10. FOOTING BOTTOM CONCRETE COVER: At the bottom of barrier footings shown throughout this Index, up to 2 inches of additional concrete cover is permitted beyond what is shown herein to accommodate soil grade irregularities.
- 11. FINISH GRADE ELEVATION: At the barrier face location, the finish grade pavement has a vertical position tolerance of ± ½" from the locations shown herein, relative to the barrier elevation. Maintain visually smooth and even pavement at the barrier face, per the approval of the Engineer.
- 12. DRAINAGE INLETS: Where called for in the Plans, install corresponding inlets per Indexes 425-030 thru 425-032.
- 13. LIGHT POLE MOUNTING: Where called for in the Plans, install aluminum light poles per Index 715-002.
- 14. OPAQUE VISUAL BARRIER: Where called for in the Plans, install Opaque Visual Barrier per Index 521-010.
- 15. BARRIER END MARKERS: For all free ends of concrete barriers that are not shielded with an end treatment or connection to another barrier or traffic railing type, install a Type 3 Object Marker on the end face per Specification 705.
- 16. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification 705. For median barriers, mount the delineator on the top of the barrier, at the centerline of barrier, with reflective sheeting facing traffic on both approaches. For shoulder barriers and split sections, mount the delineators on the top of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.

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SECTION A-A 38" HEIGHT MEDIAN BARRIER (See Sheet 3 for Steel Reinforcing Details)

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FY 2018-19 STANDARD PLANS

DOWEL DETAILS=

MEDIAN BARRIER

8. FREE ENDS: When the barrier end does not terminate with a Traffic Railing Connection, Guardrail

Connection, Crash Cushion Connection, or Sloped End Treatment as called for in the Plans, terminate in

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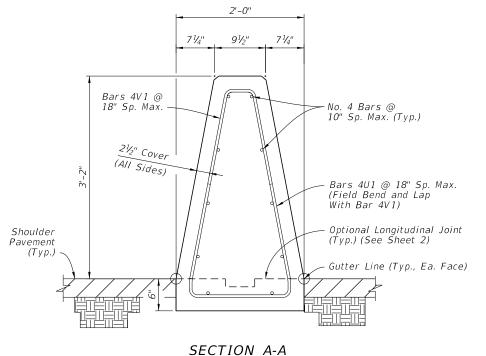
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accordance with the Free End Reinforcing detail on Sheet 3.

PLAN VIEW - 38" HEIGHT MEDIAN BARRIER FREE END REINFORCING (See Note 3)

No. 4 Bars (Only Top And Bottom Bars - Field Cut Bars 4V1 & 4U1 Shown For Clarity, \overline{A} Others Similar) 3" Cover B91/2" Bars 4V1 & 4U1 @ 18" Sp. Max. 4 Sp. @ 8" (±½") 3'-0" End Transition for Guardrail

PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (See Note 3)





Steel Qty. = 11.8 LB/FT

VIEW B-B REDUCED SECTION OF END TRANSITION FOR GUARDRAIL (End of Barrier)

1'-6"

11¾"

No. 4 Bars Tapered

with Barrier Height

Cover Varies

(Diagonal

Segment)

Shoulder

(Typ.)

Pavement

Bars 4V1

(Field Cut to Fit Vertically as Reqd. & Field Bend to

No. 4 Bars @ 10" Sp. Max.

(Field Cut To Fit

Transversally As

Reqd. & Field

Bend To Lap

With 4V1)

21/2" Cover

Lap with Bars 4U1)

NOTES:

- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 2.
- 2. BAR BENDING DIAGRAMS: For additional information on Bars 4V1 and 4U1, see the details on Sheet 22.
- 3. PLAN VIEWS: Only top and bottom longitudinal reinforcing is shown for clarity. For all longitudinal steel locations, see the section views.

MEDIAN BARRIER - REINFORCING DETAILS

2½" Cover

REVISION 11/01/17

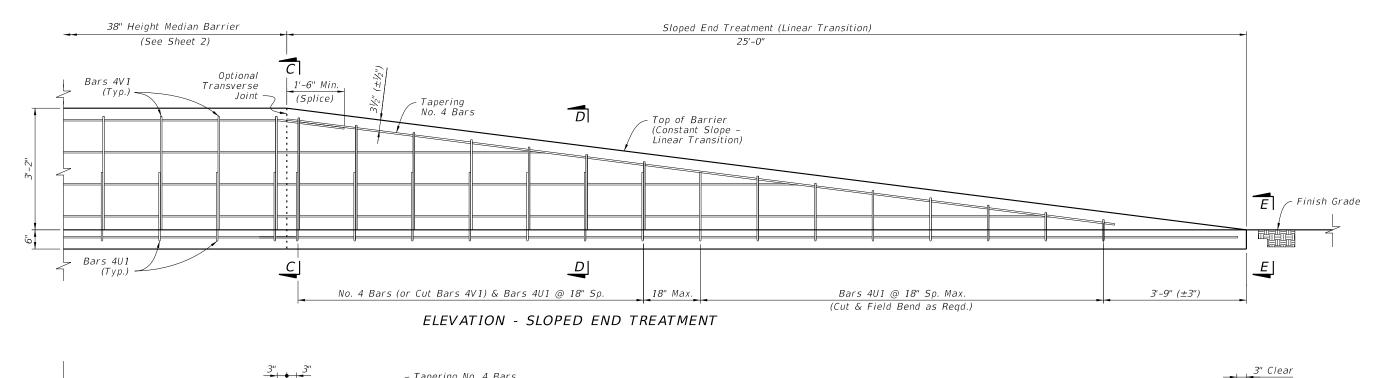
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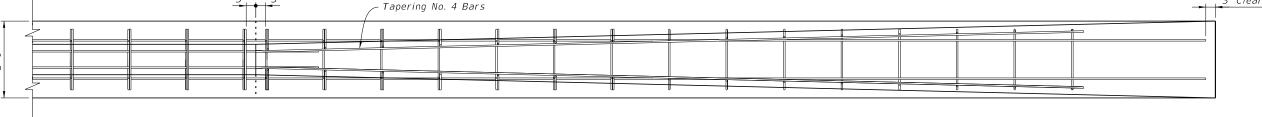
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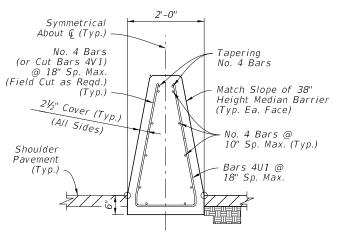
CONCRETE BARRIER

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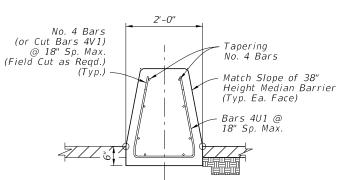




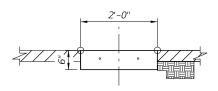
PLAN - SLOPED END TREATMENT (Only Top & Bottom Longitudinal Bars Shown for Clarity, See Section Views for All Longitudinal Steel Locations)



SECTION C-C **BEGIN TRANSITION** REINFORCING (Height Varies Linearly per Elevation View)



SECTION D-D INTERMEDIATE TRANSITION REINFORCING (Height Varies Linearly per Elevation View)



VIEW E-E **END TRANSITION**

NOTES:

- 1. GENERAL: Install Sloped End Treatment only where called for in the plans.
- 2. JOINTS: Construction or Doweled Joints are not permitted within the Sloped End Treatment segment.

MEDIAN BARRIER -SLOPED END TREATMENT

REVISION 11/01/17

DESCRIPTION:

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FY 2018-19 STANDARD PLANS

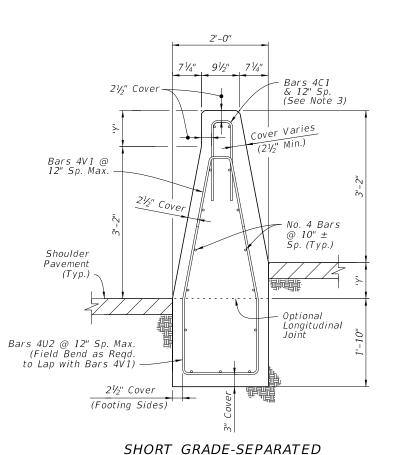
CONCRETE BARRIER

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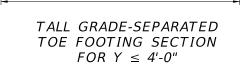


91/2" 71/4" No. 4 Bars @ 8" Sp. Max. (Typ.) 2" Cover (Barrier & Stem) Shoulder Pavement of Optional No. 5 Bars Longitudinal @ 8" Sp. (See Note 4) Shoulder Pavement Lower Limit of Optional Longitudinal Joint(s) No. 5 Bars 2" Cover 3" Cover @ 8" Sp. Max. Hook Hook (Length = 'W'-5")(Min.) (Min.) TALL GRADE-SEPARATED

HEEL FOOTING SECTION

 $FOR Y \leq 4'-0''$

2'-0"



Hook

(Min.)

No. 5 Bars

@ 8" Sp. Max.

(Length' = 'W'-5")

2'-0"

71/4" 91/2" 71/4"

2" Cover

(Barrier

& Stem)

No. 5 Bars

(See Note 4)

Shoulder

Pavement

3" Cover

@ 8" Sp.

No. 4 Bars @ 8"

Shoulder

Pavement

Sp. Max. (Typ.)

Upper Limit

of Optional

Joint(s)

Longitudinal

Lower Limit

of Optional

Joint(s)

2" Cover

Hook

(Min.)

Longitudinal

NOTES:

1. GENERAL: Install the Grade-Separated sections where shown in the Plans and as required to accommodate vertical offsets in pavement of Height Y. Doweled Joints are not permitted within Grade-Separated sections.

SECTION FOR $Y \leq 9$ "

- 2. CONNECTIONS BETWEEN DIFFERENT SECTIONS: Connect Short Grade-Separated sections and Tall Grade-Separated sections using a continuous pour or Transverse Joint, where longitudinal steel that aligns within the adjacent section is maintained continuously between sections or has a full lap splice with the adjacent section's longitudinal steel. Connect Short Grade-Separated sections and 38" Height Median Barrier sections of Sheet 2 using a ¾" Doweled Joint.
- 3. SHORT GRADE-SEPARATED SECTIONS: Bars 4C1 and the two uppermost longitudinal bars may be omitted for segments where $\rm Y < 2^n$.
- 4. TALL GRADE-SEPARATED SECTIONS: For the vertical and transverse steel reinforcement shown in the Tall Grade-Separated Sections, bar bending diagrams are not provided due to varying section dimensions and Longitudinal Joint locations. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

Longitudinal Joints are permitted between the vertical limits shown, and must remain level and at a consistent height per each continuous casting of concrete. Longitudinal Joints may change elevations at Transverse Joint locations. Field bending of bars is permitted at Longitudinal Joint locations.

Transverse Joints between Tall Grade-Separated Sections do not require continuous steel across the joint if the following conditions are met:

- i. The barrier length on both sides is at least 40 feet, where each segment has continuous steel reinforcement.
- ii. The barrier's vertical steel spacing is reduced to 4" O.C. for a total of 12 spaces on both sides of the joint.

Grade separation Heights of $Y \leq 9$ " are permitted on a limited basis using the Tall Grade–Separated section; this is to accommodate cases where maintaining the spread footing through lower height segments is more practical than changing to the Short Grade–Separated section.

TALL GRADE-SEPARATED SECTIONS DIMENSION TABLE Max. Height, Y 1'-0" 1'-6" 2'-0" 2'-6" 3'-0" 3'-6" 4'-0" Footing Width, W 3'-3" 3'-6" 3'-9" 4'-0" 4'-3" 4'-6" 4'-6"

MEDIAN BARRIER - GRADE-SEPARATED

LAST REVISION 15/11/01/17

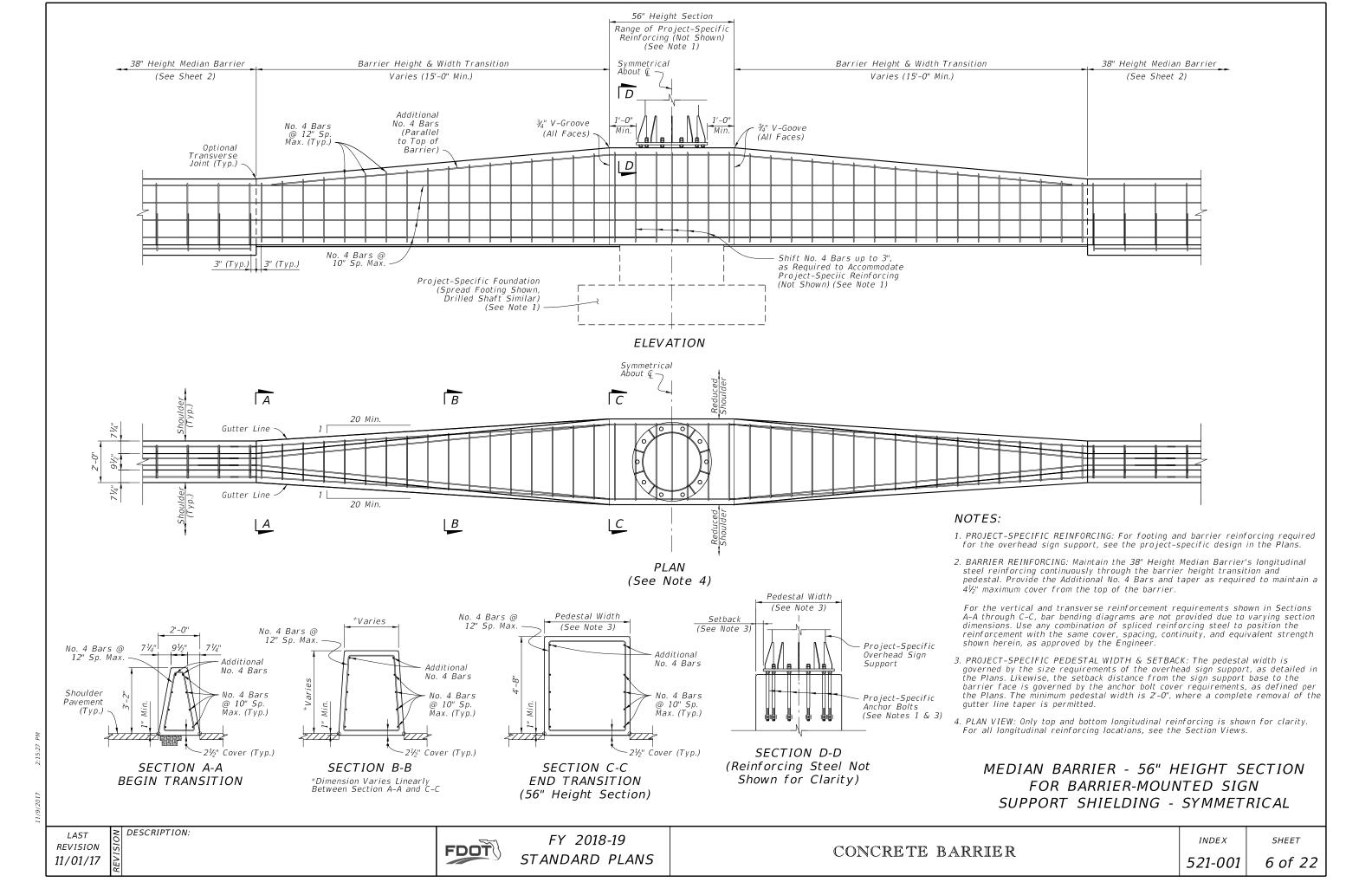
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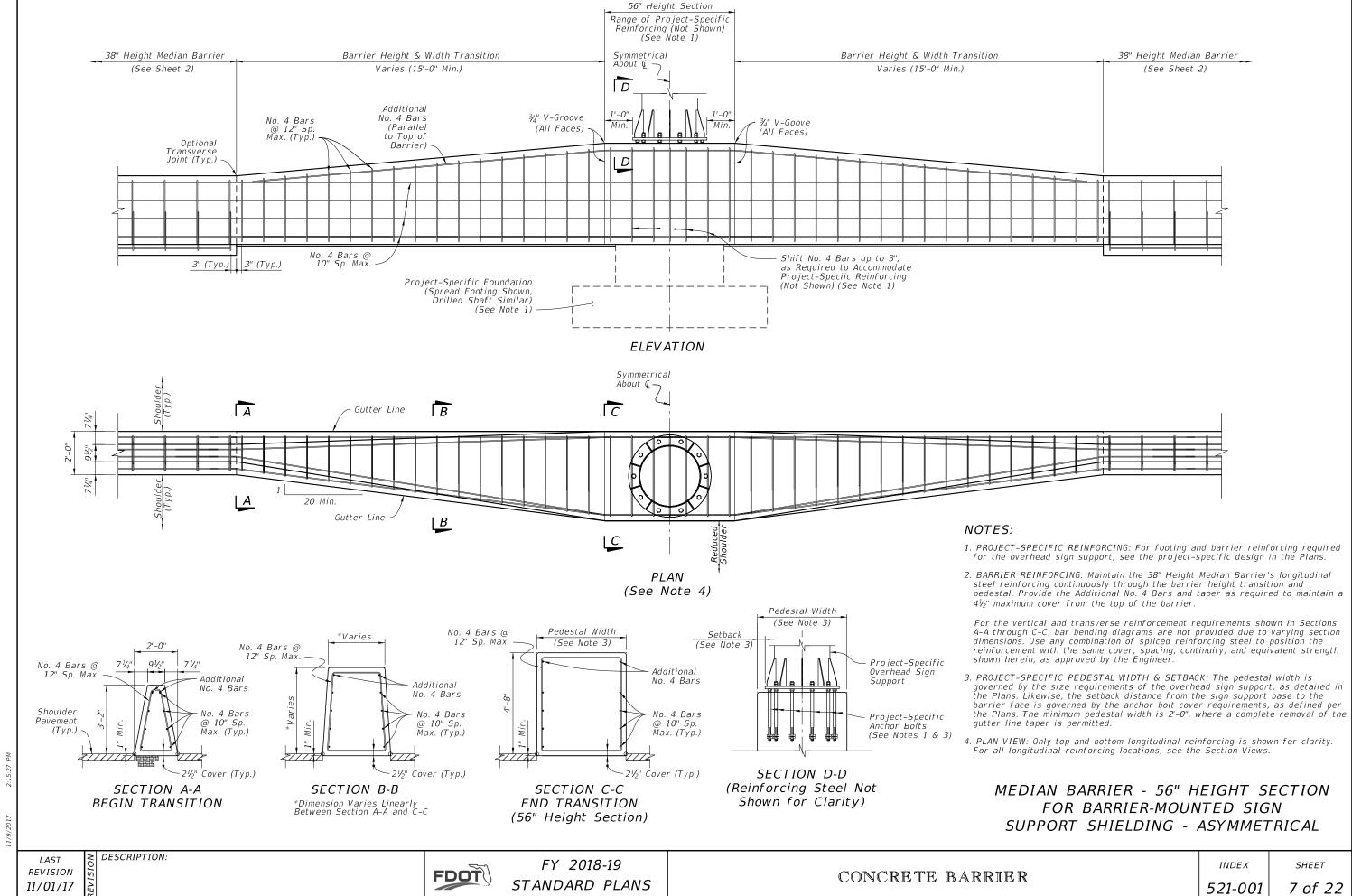
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CONCRETE BARRIER

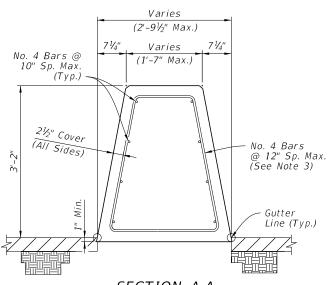
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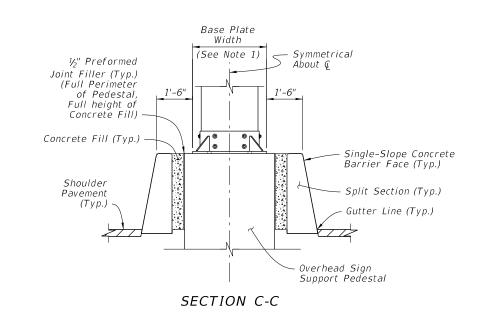
PLAN(See Note 4)





1'-3" 71/4" 73/4" No. 4 Bars @ 10" Sp. Max. (Roadway Side) (Typ.)No. 4 Bars @ 15" Sp. Max. (Back Side) 21/2" Cover (Typ.)(All Sides) No. 4 Bars @ 12" Sp. Max. (See Note 3)

SECTION B-B 38" HEIGHT SPLIT SECTION (OPPOSITE SIDE SIMILAR BY OPPOSITE HAND)



NOTES:

- 1. OVERHEAD SIGN SUPPORT: The overhead sign support shown is an example only; see the Plans for the actual shape dimensions and requirements. The overall length and width of the split barrier system is governed by the project-specific overhead sign support dimensions, as defined in the Plans.
- 2. MULTIPLE SIGN SUPPORTS: The parallel segment may be lengthened to accommodate multiple sign supports, with the approach and trailing tapers located 1 foot, measured longitudinally, upstream and downstream from the first and last sign support bases, respectively.
- 3. STIRRUP BARS: For the vertical and transverse reinforcement requirements shown in Sections A-A and B-B, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.
- 4. PLAN VIEW: Only outermost longitudinal reinforcing is shown for clarity. For all longitudinal reinforcing locations, see the Section Views.

MEDIAN BARRIER - 38" HEIGHT SPLIT SECTION FOR STAND-ALONE SIGN SUPPORT SHIELDING

REVISION 11/01/17

DESCRIPTION:

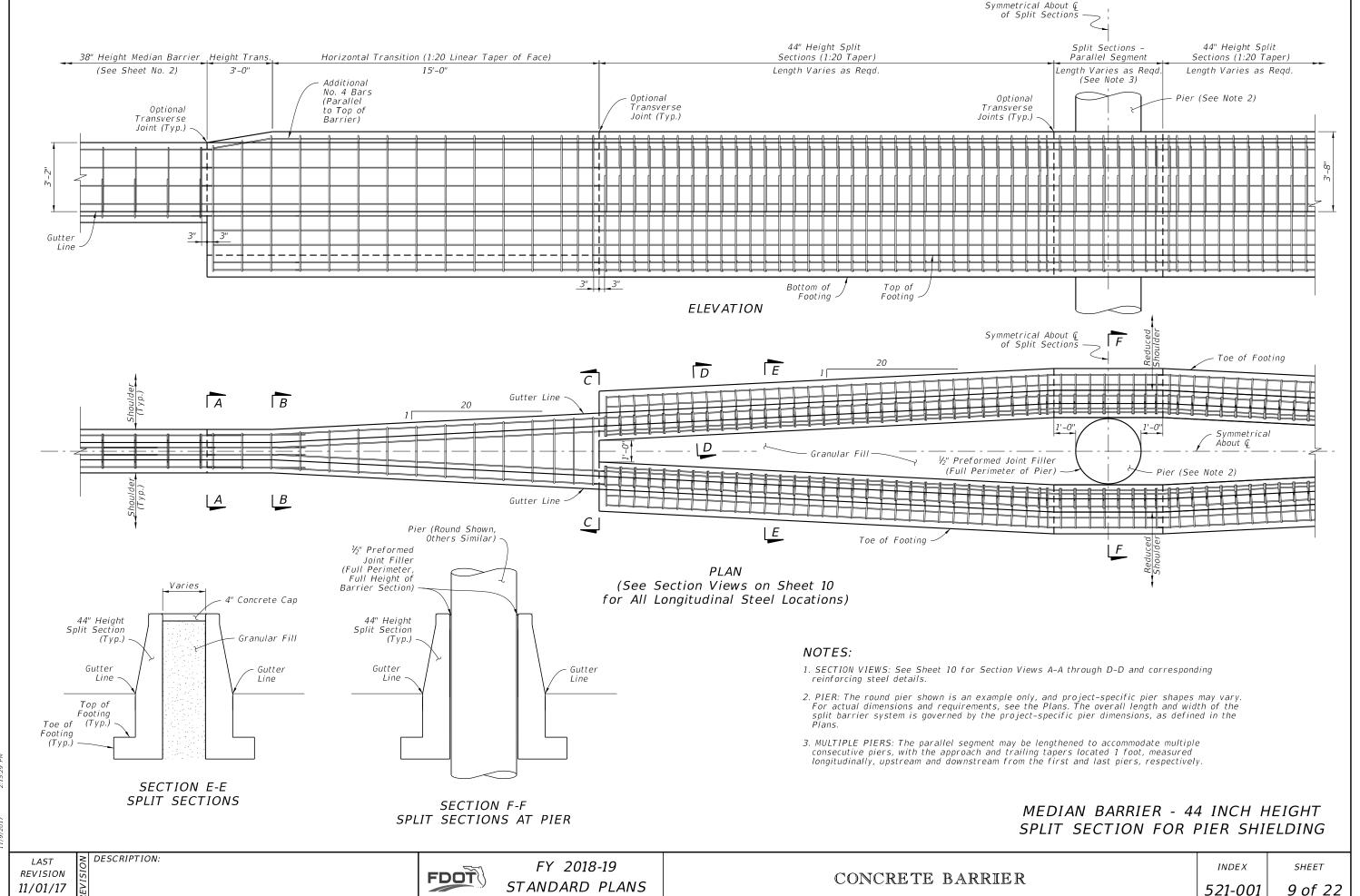
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CONCRETE BARRIER

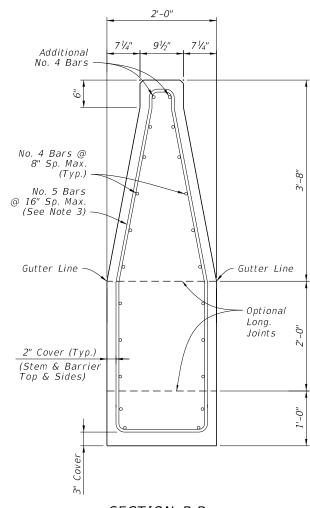
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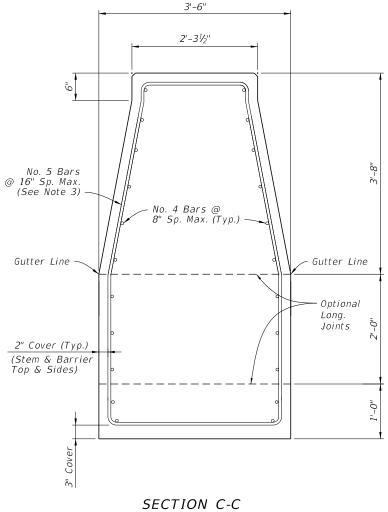


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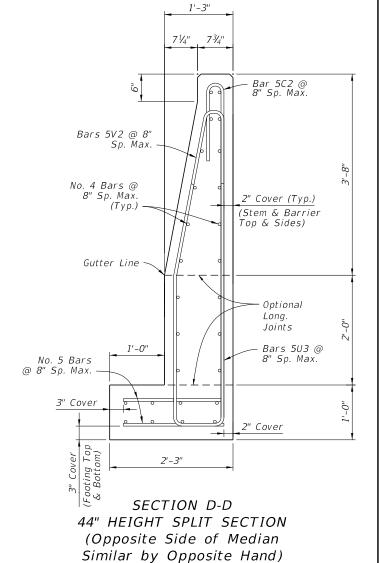
SECTION A-A BEGIN HEIGHT TRANSITION (show spliced bars)



SECTION B-B END HEIGHT TRANSITION BEGIN WIDTH TRANSITION



SECTION C-C END WIDTH TRANSITION BEGIN SPLIT SECTIONS



Concrete Qty. = 0.30 CY/FT Stee/ Qty. = 52.6 LB/FT

NOTES:

DESCRIPTION:

- 1. GENERAL: Work with the Plan and Elevation views on Sheet 9.
- 2. LONGITUDINAL REINFORCING CONTINUITY: Maintain all longitudinal steel reinforcing shown in Section C-C continuously into Section D-D (spliced where required). The additional longitudinal reinforcing shown in Section D-D does not require continuity into Section C-C, and it starts 3" from the construction joint or edge of concrete per the details on Sheet 9.
- 3. STIRRUP BARS: For the vertical and transverse reinforcement requirement shown, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

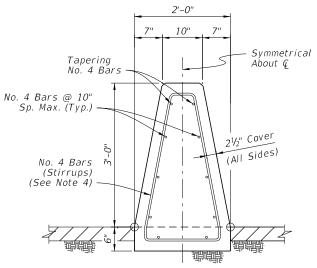
MEDIAN BARRIER - 44 INCH HEIGHT SPLIT SECTION PIER SHIELDING - DETAILS

LAST REVISION 11/01/17

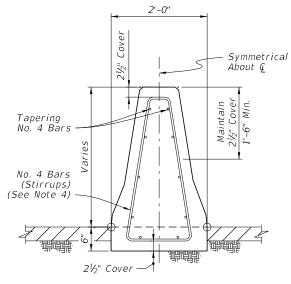
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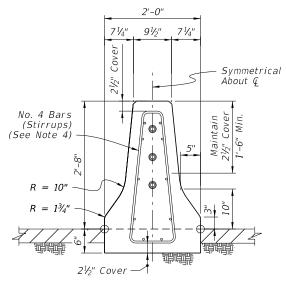
SECTION A-A BEGIN TRANSITION - OPTION 'A' MATCH SINGLE-SLOPE 38" HEIGHT MEDIAN BARRIER



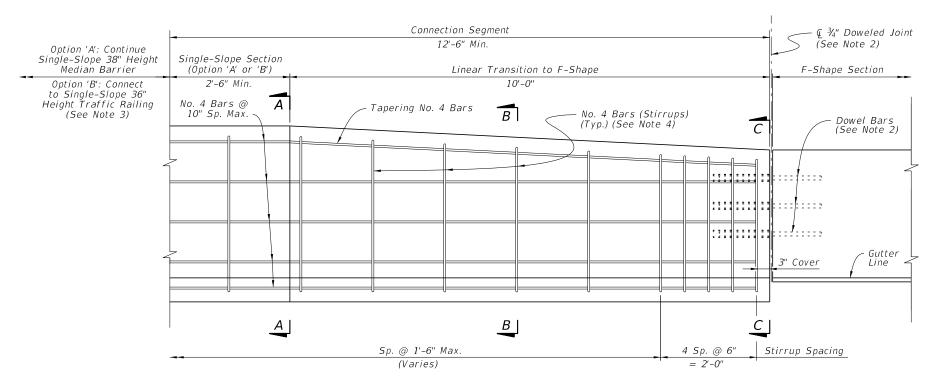
SECTION A-A BEGIN TRANSITION - OPTION 'B' MATCH SINGLE-SLOPE 36" HEIGHT TRAFFIC RAILING (Bridge Applications)



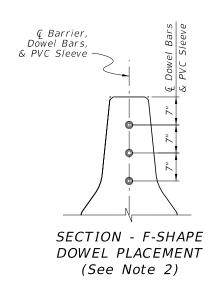
SECTION B-B INTERMEDIATE SECTION OF LINEAR TRANSITION



SECTION C-C **END TRANSITION** MATCH 32" HEIGHT F-SHAPE SECTION



ELEVATION (Reverse Direction Similar by Opposite Hand)



NOTES:

- 1. GENERAL: Construct the Connection Segment as required per the Plans to connect existing F-Shape sections to Single-Slope Median Barrier or Traffic Railing sections. Construct Option 'A' or 'B' as required to match the heights of the connecting sections.
- 2. DOWELED JOINT: Install Dowel Bars per the Dowel Details on Sheet 2.
- 3. TRAFFIC RAILING CONNECTION: For the Option 'B' connection, use a Doweled Joint per Sheet 2 and the additional Free End Reinforcing with reduced bar spacing per Sheet 3.
- 4. STIRRUP BARS: For the vertical and transverse reinforcement requirements shown, bar bending diagrams are not provided due to varying section dimensions. Use any combination of spliced reinforcing steel to position the reinforcement with the same cover, spacing, continuity, and equivalent strength shown herein, as approved by the Engineer.

MEDIAN BARRIER - CONNECTION TO F-SHAPE

REVISION 11/01/17

FDOT

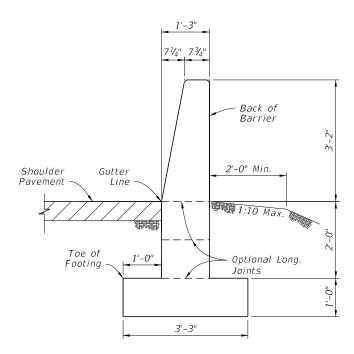
FY 2018-19 STANDARD PLANS

CONCRETE BARRIER

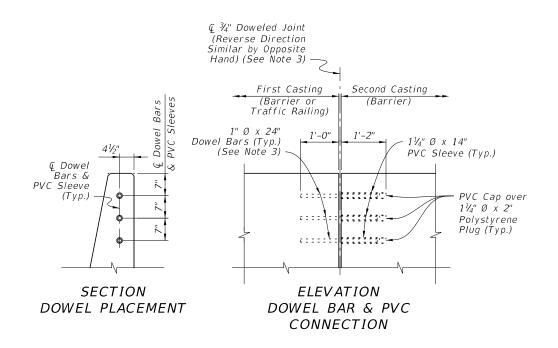
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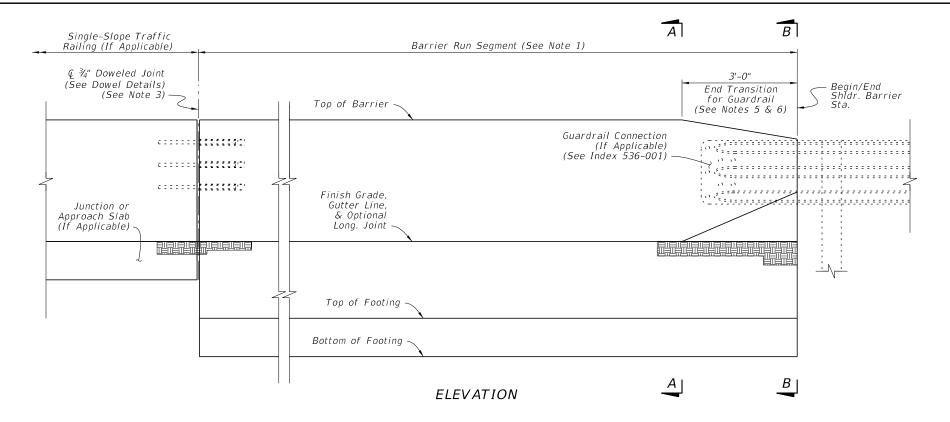
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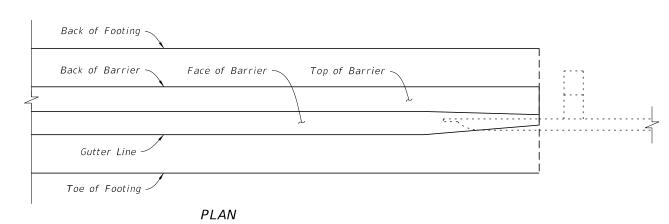


SECTION A-A 38" HEIGHT SHOULDER BARRIER (See Sheet 13 for Reinforcing Steel Details)



DOWEL DETAILS





NOTES:

- 1. BARRIER RUN SEGMENT: Either the 38" Height Shoulder Barrier or the differing Shoulder Barrier sections shown throughout the Index may be placed within this segment as required per the Plans.
- 2. SECTION VIEWS: For additional Views A-A and B-B, see Sheet 13.
- 3. DOWELED JOINTS: See the General Notes on Sheet 1 for usage of joint types. Place steel reinforcing with a longitudinal 3" cover adjacent to the joint face in the barrier. Use ASTM A36 smooth round bars with hot-dip galvanization.

For the dowel connection into the first casting, the dowel may be cast-in-place for new concrete or placed into a $1\frac{1}{6}$ " \times x 13" (\pm $\frac{1}{2}$ ") drilled hole for cured concrete. For drilled holes larger than 11/8"O, secure the dowel with adhesive in accordance with Specification Section 416. No load testing is required.

For the dowel connection into the second casting, use a 1½" NPS Schedule 80 PVC pipe with a sealed cap, cast-in-place as shown.

- 4. TRAFFIC RAILING CONNECTIONS: Align the barrier and Traffic Railing faces and connect with the ¾" Doweled Joint.
- 5. GUARDRAIL CONNECTIONS: Connect Guardrail using the Transition Connections to Rigid Barrier per Index 536-001 in conjunction with the 16'-0" End Segment for Guardrail shown herein.
- 6. CRASH CUSHION CONNECTIONS: Connect Crash Cushions per Index 544-001 in conjunction with the 3'-0" End Transition for Guardrail as shown herein.
- 7. FREE ENDS: When the barrier end does not terminate with a Traffic Railing Connection, Guardrail Connection, or Crash Cushion Connection as called for in the Plans, terminate in accordance with the Free End Reinforcing Note on Sheet 13.

SHOULDER BARRIER

LAST **REVISION** 11/01/17

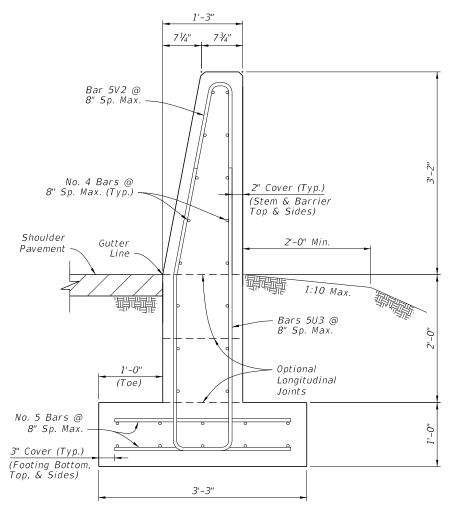
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CONCRETE BARRIER

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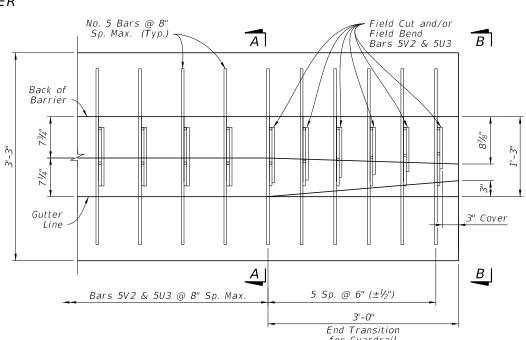


SECTION A-A 38" HEIGHT SHOULDER BARRIER

Concrete Qty. = 0.32 CY/FT Steel Qty. = 50.9 LB/FT

NOTES:

- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 12. The Section Option footings shown on Sheet 14 may be substituted where called for in the Plans.
- 2. FREE END REINFORCING: Where shown in the Plans, terminate the 38" Height Barrier section with a transverse vertical end face. Reduce the spacing of Bars 5V2 and 5U3 to 6" for 5 Spaces, placed with 3" cover from the barrier's end face.
- 3. BAR BENDING DIAGRAMS: For additional details for bars 5V2 and 5U3, see the Bar Bending Diagrams on Sheet 22.



VIEW B-B REDUCED SECTION OF **END TRANSITION** FOR GUARDRAIL (End of Barrier)

3'-3"

6½"

Bars 5V2 @ 6" Sp. (Field Bend Top & Cut Bottom to Lap

Shoulder

Pavement

No. 5 Bars @ 6" Sp.

3" Cover (Typ.)

(Footing Bottom,

Top, & Sides)

with Bars 5U3)

Cover Varies | |

21/2" Cover

2" Cover (Min.)

1'-0"

(Toe)

87/8"

No. 4 Bars Tapered Down with Barrier Height

No. 4 Bars

2'-0" Min.

Optional Property

Joints.

Longitudinal

@ 8" Sp. Max. (Typ.)

1:10 Max.

Bars 5U3 @ 6" Sp. (Field Bend Bottom

to Align with Bars 5V2)

2" Cover

PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (Longitudinal Steel Not Shown for Clarity)

SHOULDER BARRIER - REINFORCING DETAILS

LAST **REVISION** 11/01/17

DESCRIPTION:

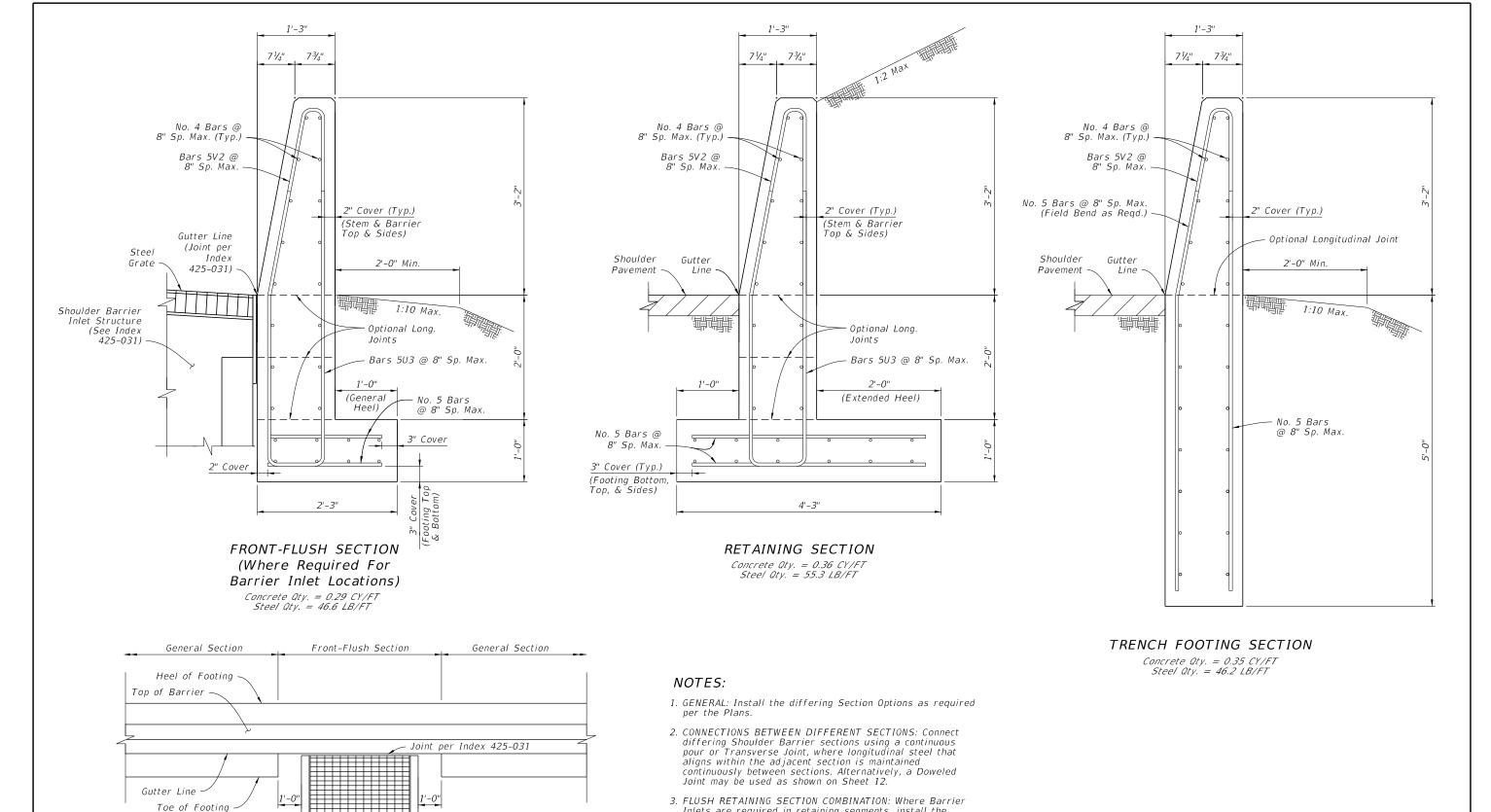
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CONCRETE BARRIER

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FRONT-FLUSH SECTION - PLAN VIEW (Not Applicable for Trench Footing Sections)

Flush Section, except replace the 1'-0" General Heel with the 2'-0" Extended Heel as shown in the Retaining Section.

Inlets are required in retaining segments, install the Use longer lateral reinforcing bars of 2'-10" length to maintain the cover shown.

SHOULDER BARRIER - SECTION OPTIONS

LAST **REVISION** 11/01/17

DESCRIPTION:

FDOT

Shoulder Barrier

per Index 425-031

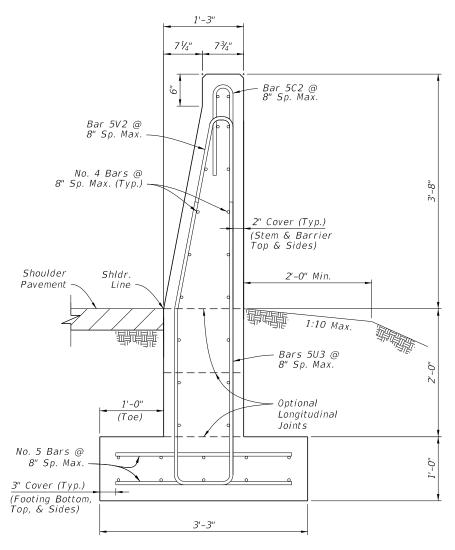
Inlet Structure

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CONCRETE BARRIER

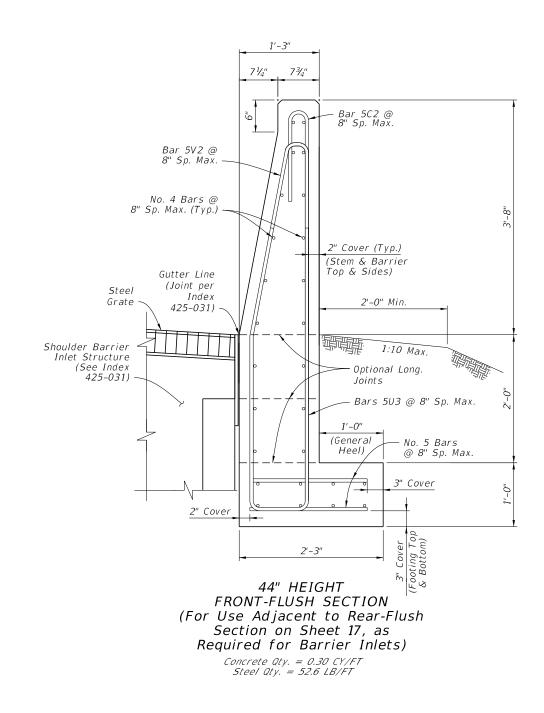


44" HEIGHT SECTION (For Use Adjacent to Rear-Flush Section on Sheet 17)

Concrete Qty. = 0.34 CY/FT Steel Qty. = 56.8 LB/FT

NOTE:

1. GENERAL: See the applicable Notes on Sheet 14.



SHOULDER BARRIER - SECTION OPTIONS (CONTINUED)

LAST **REVISION** 11/01/17

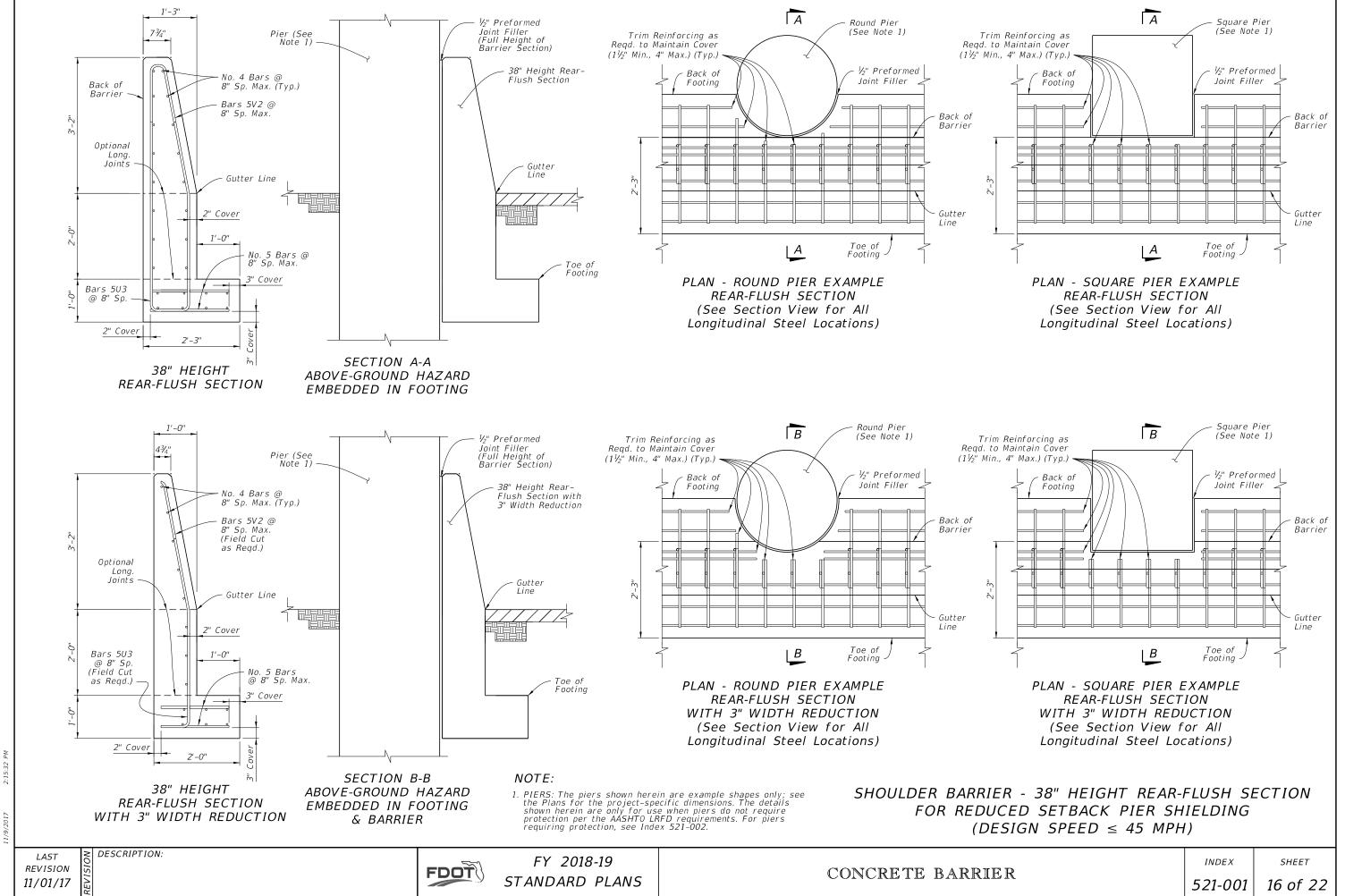
DESCRIPTION:

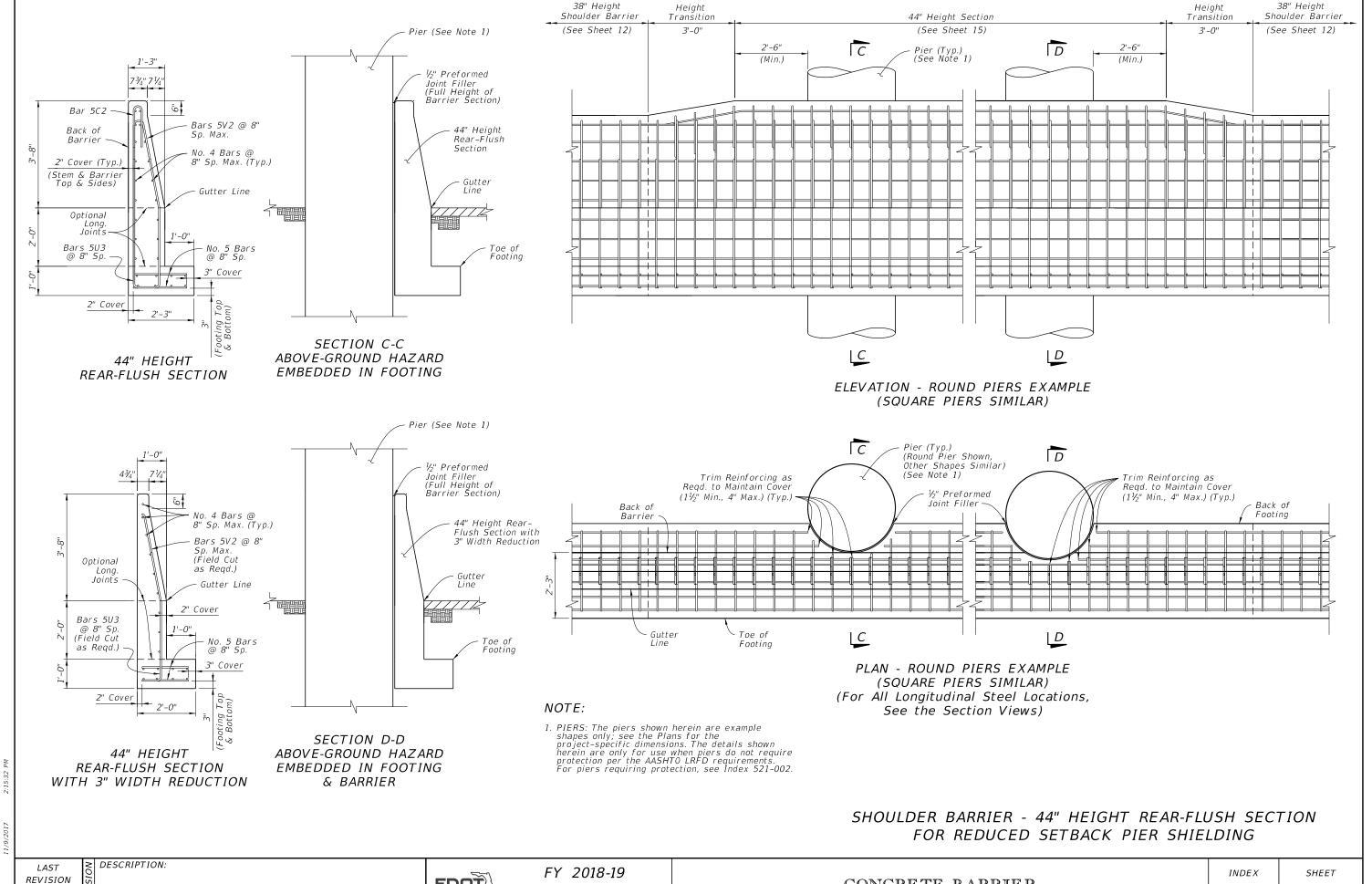
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CONCRETE BARRIER

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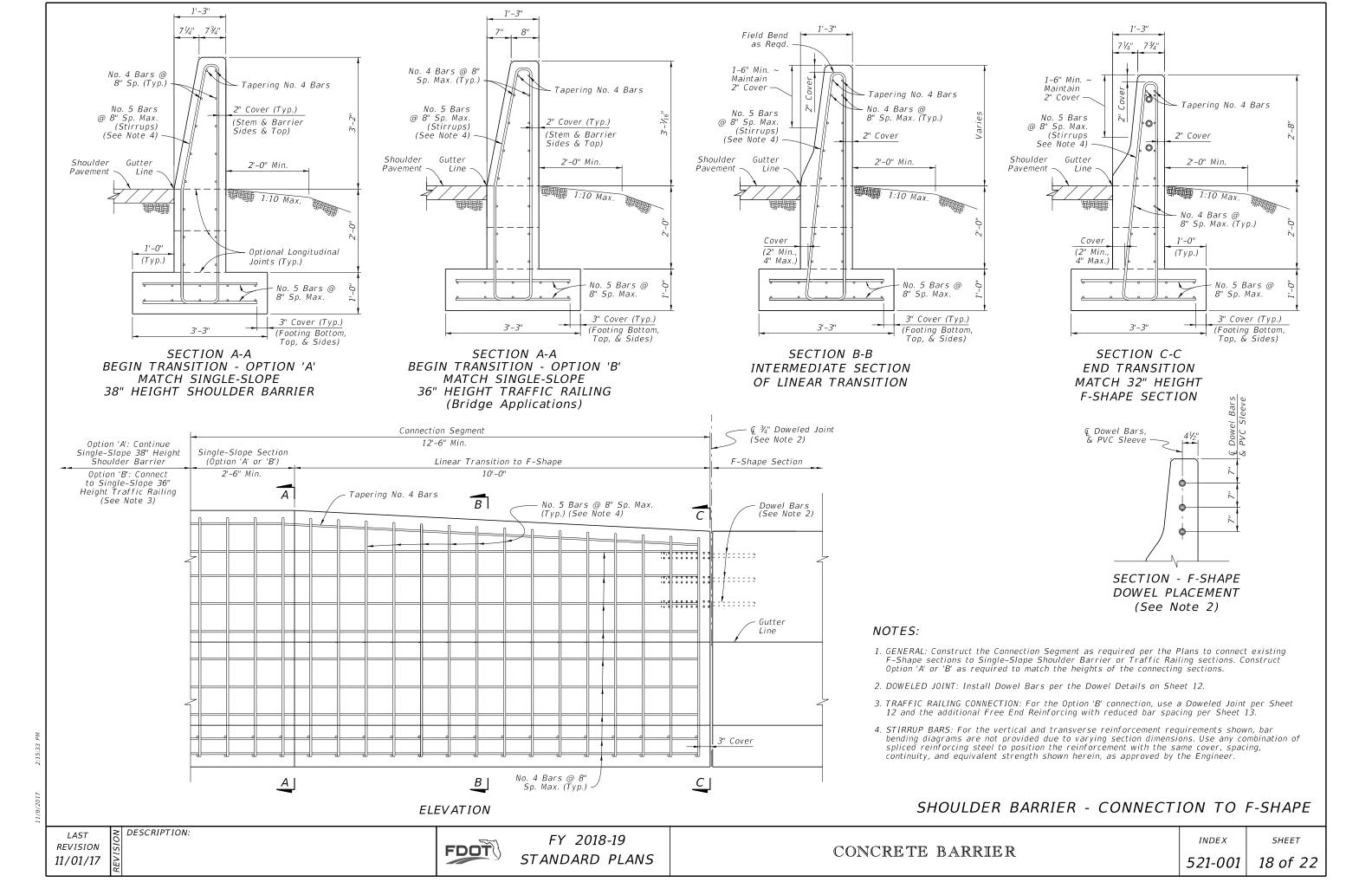


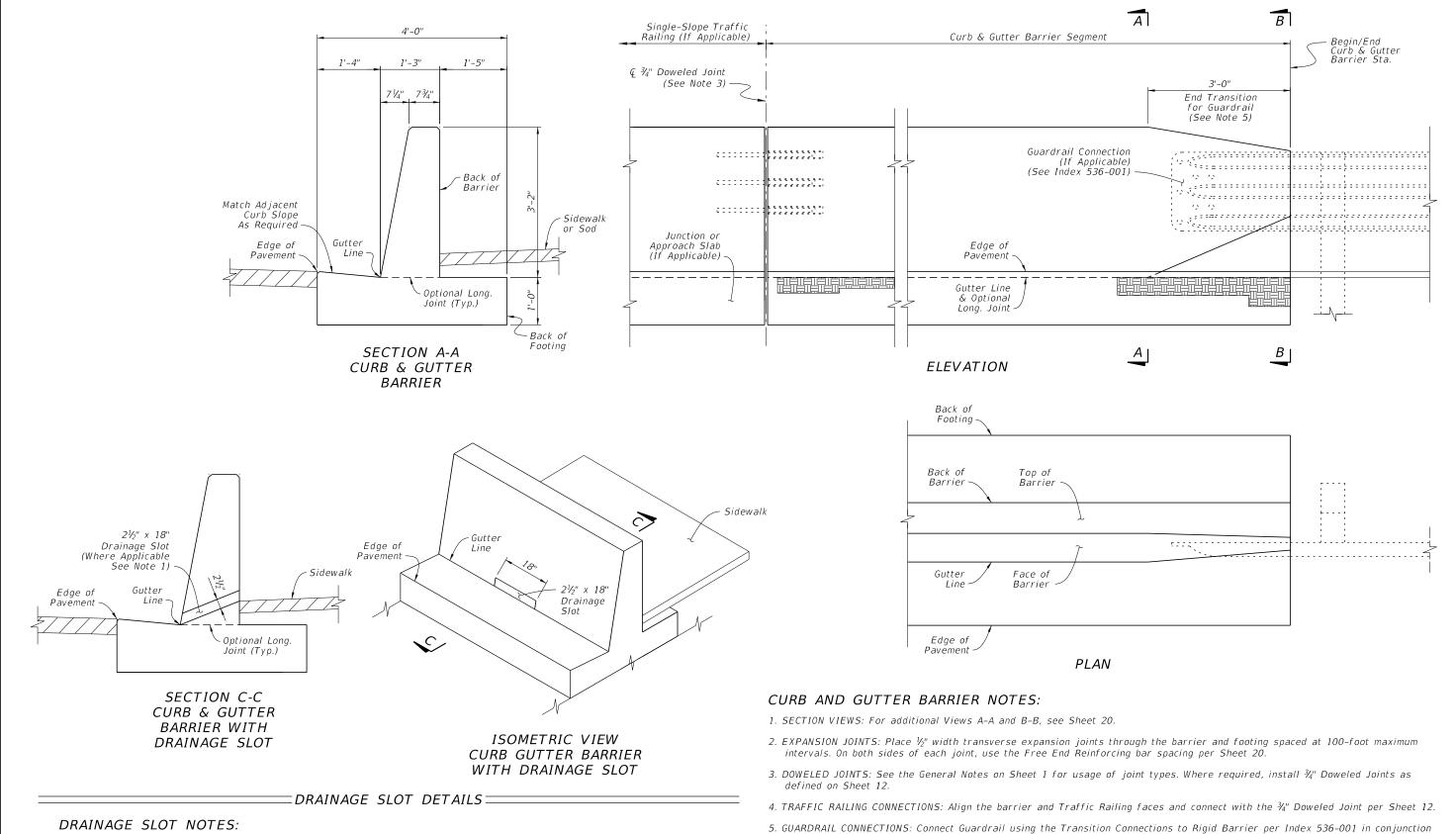
38" Height

11/01/17

FDOT

38" Height





- 1. GENERAL: Place $2\frac{1}{2}$ " x 18" Drainage Slots at locations and/or spacing called for in the Plans.
- 2. STEEL REINFORCEMENT CONFLICT: When the Drainage Slot encounters a conflict with reinforcing steel, shift or cut the reinforcing steel to provide $2\frac{1}{2}$ "(± $\frac{1}{2}$ ") of concrete cover for the reinforcing around the Drainage Slot. If cutting the vertical bars, maintain 8" bar spacing. If shifting the vertical bars, move the bars from the standard 8" spacing location to the closest end of the drainage slot (distributing additional vertical reinforcement evenly on each side of the Drainage Slot).
- with 3'-0" End Transition for Guardrail as shown herein.
- 6. FREE ENDS: When the barrier end does not terminate with a Traffic Railing connection or Guardrail connection as called for in the Plans, terminate the barrier in accordance with the Free End Reinforcing Note on Sheet 20.

CURB AND GUTTER BARRIER

REVISION 11/01/17

DESCRIPTION:

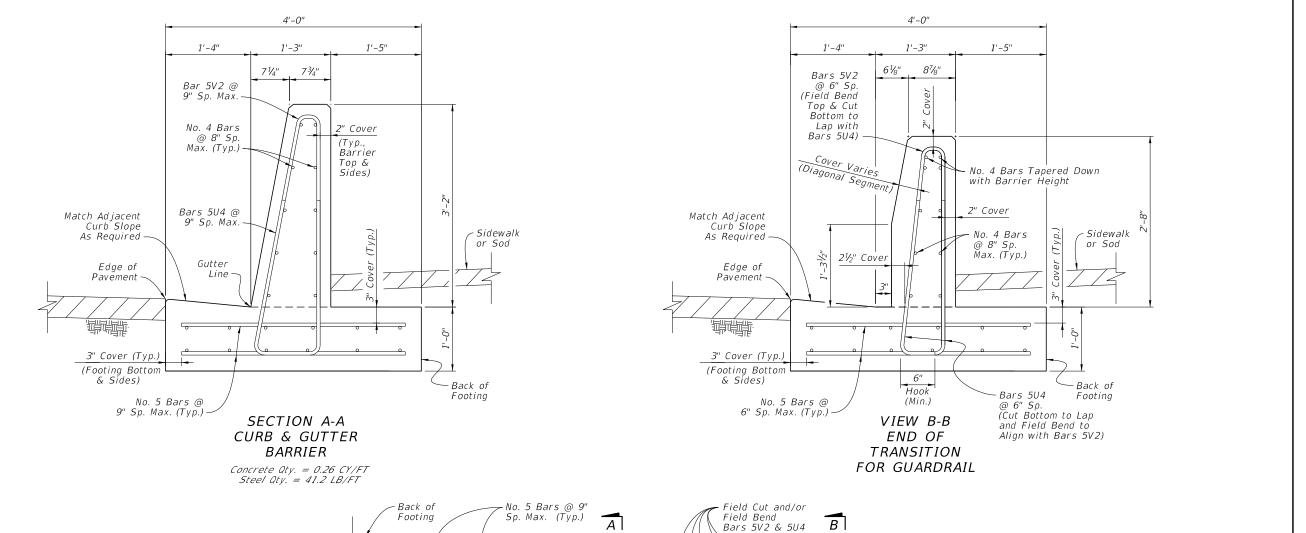
FDOT

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CONCRETE BARRIER

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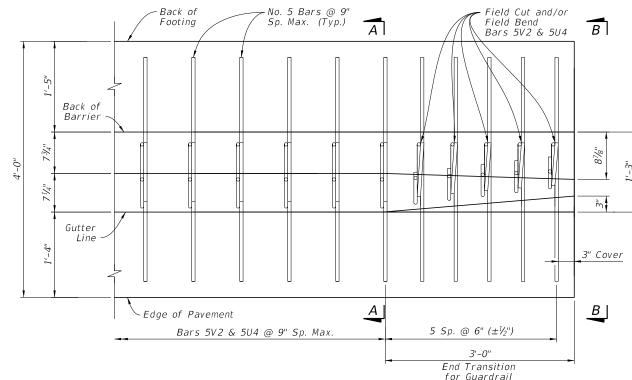
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DESCRIPTION:

- 1. GENERAL: Work with the Plan and Elevation Views on Sheet 19.
- 2. FREE END REINFORCING: Where shown in the Plans, terminate the 38" Curb & Gutter Barrier section with a transverse vertical end face. Reduce the spacing of Bars 5V2 and 5U4 to 6" for 5 Spaces, placed with 3" cover from the barrier's
- 3. BAR BENDING DIAGRAMS: For additional details for bars 5V2 and 5U4, see the Bar Bending Diagrams on Sheet 22.



PLAN VIEW - END SEGMENT FOR GUARDRAIL CONNECTION (Longitudinal Steel Not Shown for Clarity)

CURB AND GUTTER BARRIER -REINFORCING DETAILS

11/01/17

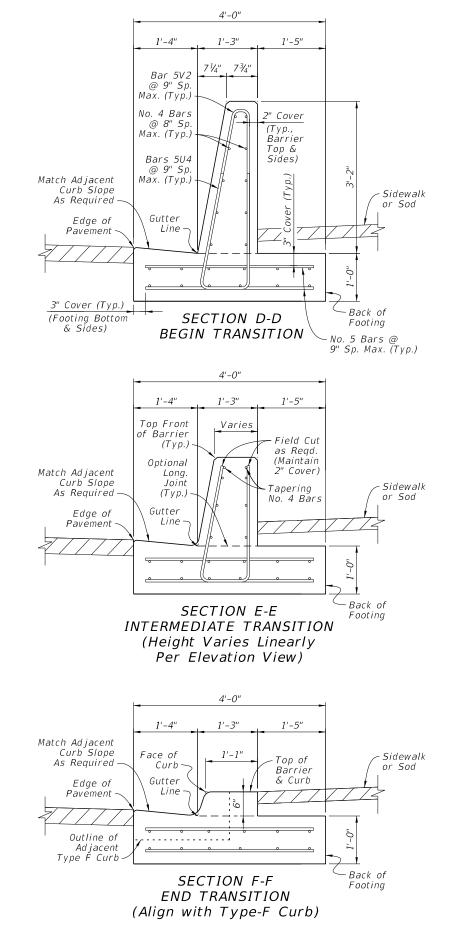
FDOT

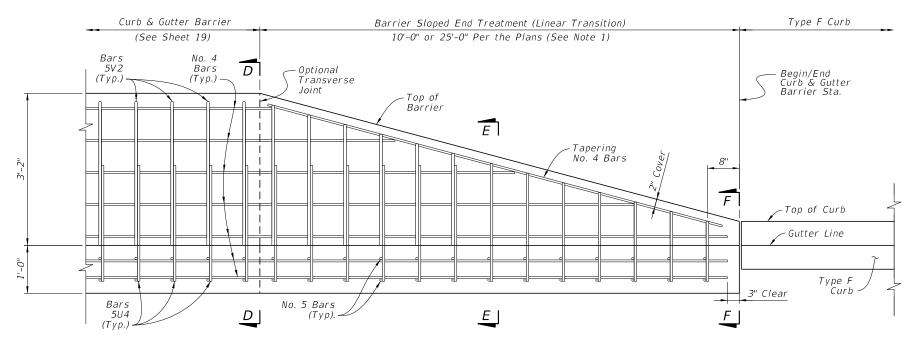
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CONCRETE BARRIER

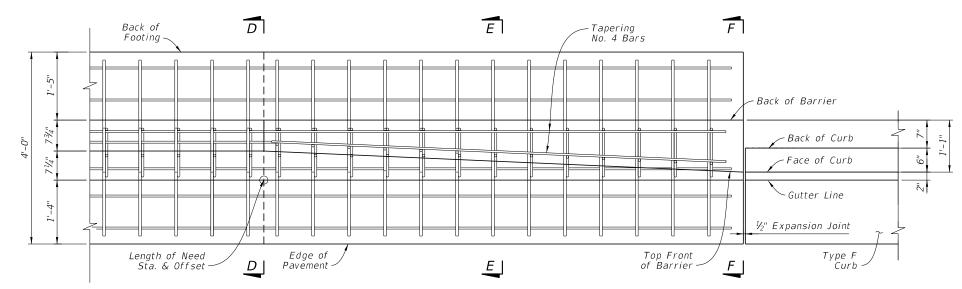
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ELEVATION - CURB AND GUTTER BARRIER SHOWING SLOPED END TREATMENT (Approach and Trailing End Similar by Opposite Hand)



PLAN - CURB AND GUTTER BARRIER SHOWING SLOPED END TREATMENT (Approach and Trailing End Similar by Opposite Hand; See Sections for All Longitudinal Steel Locations)

NOTES:

- 1. GENERAL: Install a Sloped End Treatment only where called for in the Plans, using either a 10'-0" length or 25'-0" length treatment as specified in the Plans. The 10'-O" length option is shown herein, while the 25'-O" length option requires additional trimmed Bars 5U4 & 5V2 at the same 9" longitudinal spacing.
- 2. BAR BENDING DIAGRAMS: For additional details on Bars 5V2 & 5U4, see the Bar Bending Diagrams on Sheet 22.

CURB AND GUTTER BARRIER -SLOPED END TREATMENT

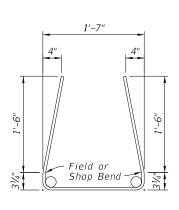
REVISION 11/01/17

DESCRIPTION:

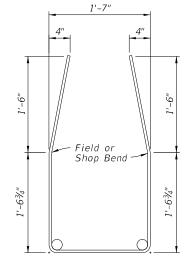
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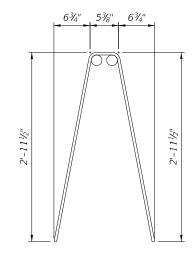
BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
C1	4	3'-8"		
C2	5	3'-0"		
U1	4	5'-1"		
U2	4	7'-8"		
U3	5	9'-7"		
U4	5	5'-9"		
V1	4	6'-4"		
V2	5	6'-3"		



BARS 4U1

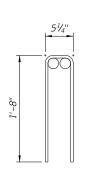


BAR 4U2



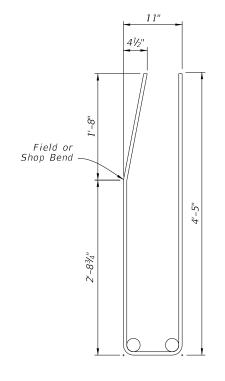
BAR 4V1

BAR 4C1

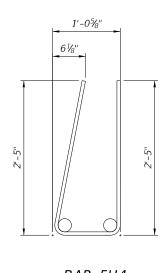


NOTES:

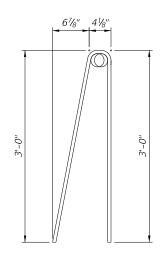
- 1. Work with the Standard Bar Bending Details per Index 415-001.
- 2. All bar dimensions in the bending diagrams are out to out.



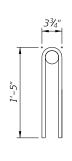
BAR 5U3



BAR 5U4



BAR 5V2



BAR 5C2

REINFORCING BAR BENDING DIAGRAMS

REVISION 11/01/17

FDOT

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CONCRETE BARRIER

INDEX *521-001*

DESCRIPTION:

SHEET NO.	CONTENTS	
1	1 Index Contents; General Notes	
2	Example Layouts – Footing Placement and Connections	
3	Barrier Plan and Elevation – Connection to Concrete Barrier – Connection to Guardrail	
4	Barrier Details – Connection to Concrete Barrier	
5	Barrier Details – Connection to Guardrail	
6	Barrier Footing Options	
7	Crash Wall Details	
8	Reinforcing Bar Bending Diagrams	

GENERAL NOTES:

- 1. CONCRETE: Use Class III or IV concrete unless otherwise called for in the Plans.
- 2. CONSTRUCTION JOINTS: Maintain continuity of reinforcement steel across Construction Joints; reinforcement lap splices are permitted immediately adjacent to joints. Construct all Pier Protection Barrier continuously, with no expansion or contraction joints. Construction Joints are classified herein as Transverse Joints or Longitudinal Joints.

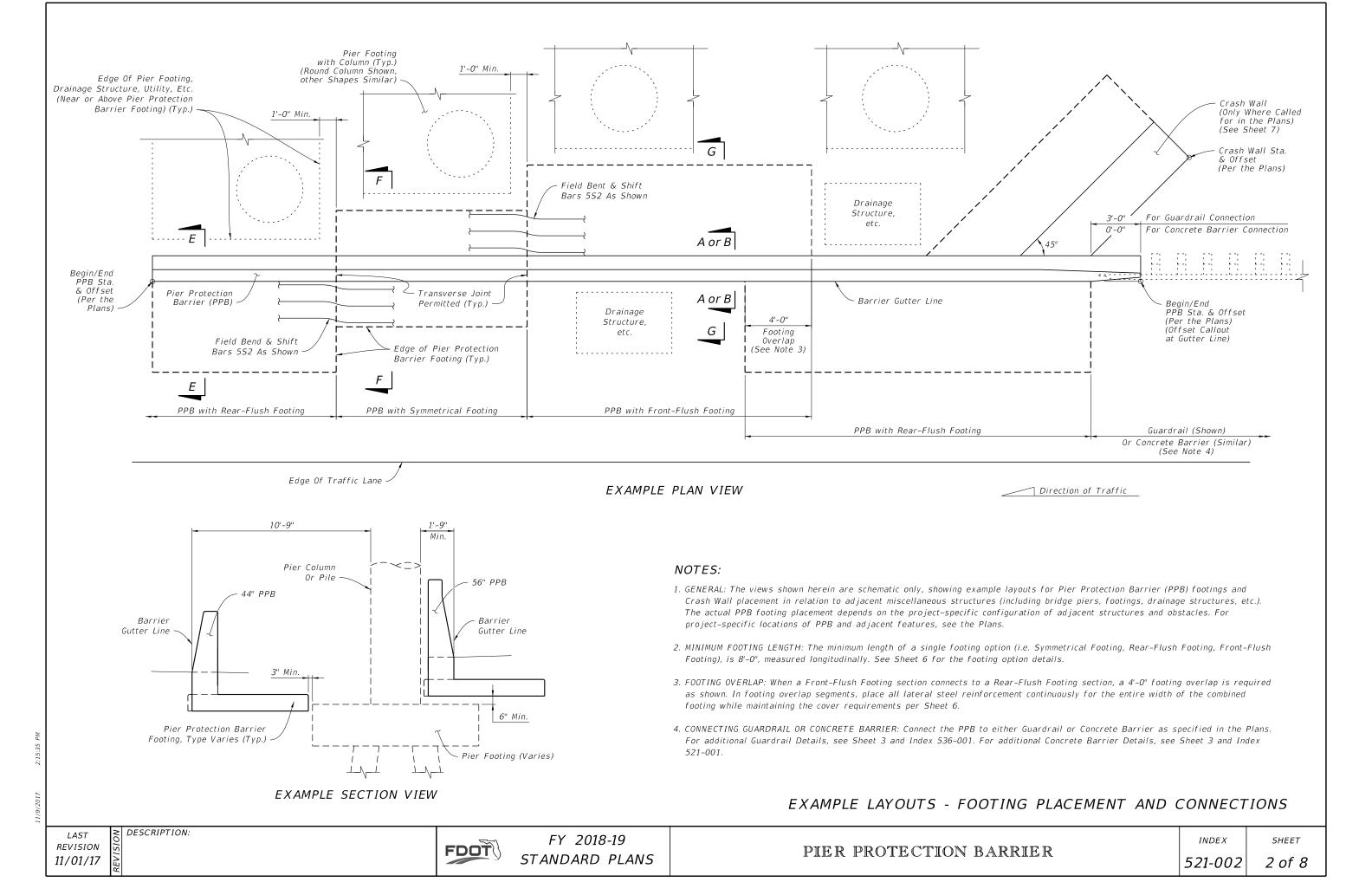
Transverse Joints are permitted at 40 foot or greater intervals along the barrier.

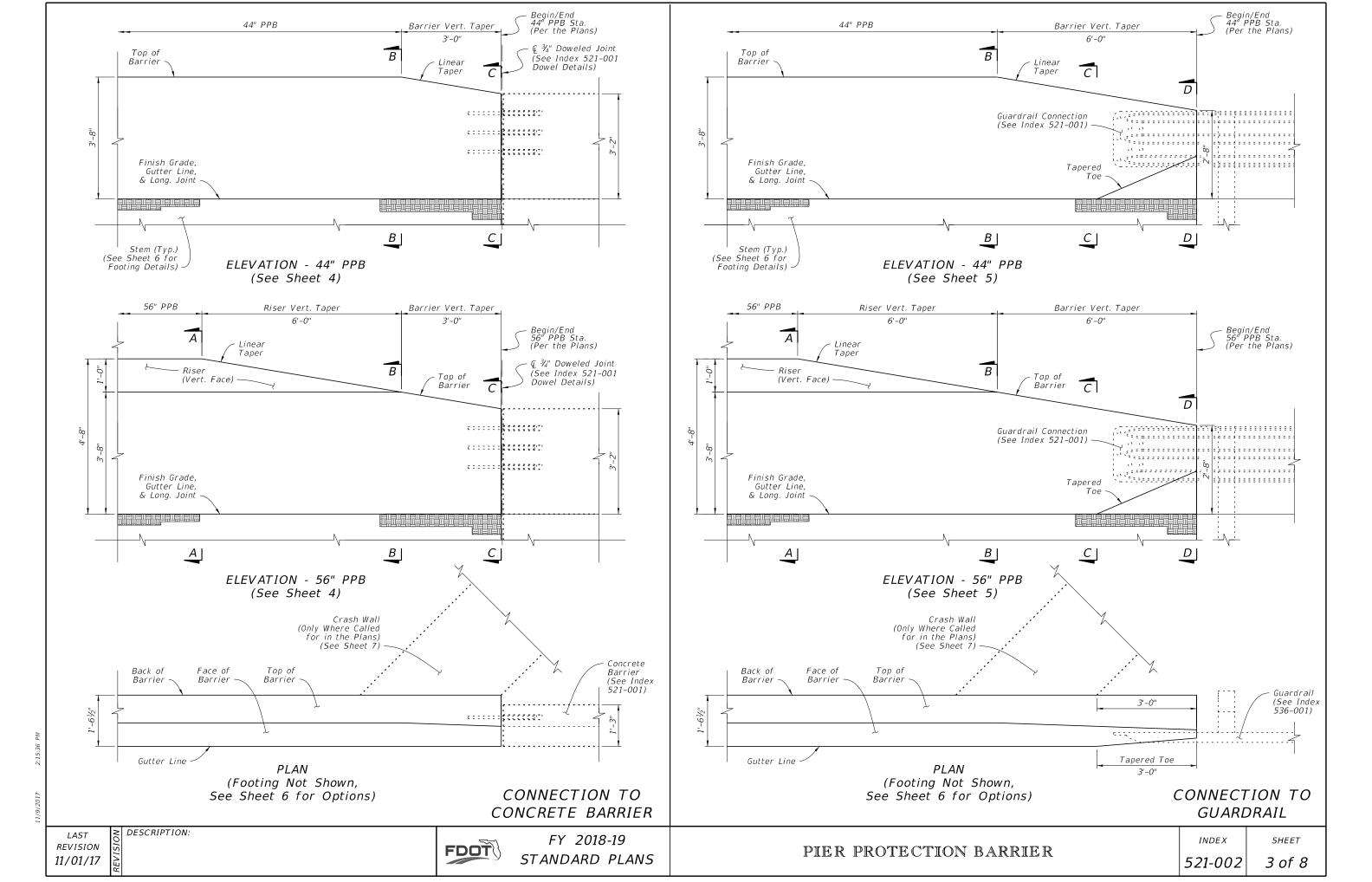
Longitudinal Joints may only be installed where indicated in the following details and notes, with a location tolerance of \pm 1" from the locations shown.

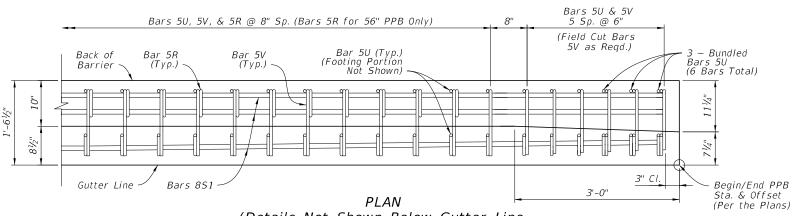
- 3. SUBGRADE: Compact the top layer of subgrade with Type B Stabilization, LBR 40 (12 in.).
- 4. DRAINAGE INLETS: See Index 425-001 for Shoulder Barrier Inlets, and isolate these structures from Pier Protection Barriers and Footings with 1" Preformed Joint Filler.
- 5. BARRIER END MARKERS: For all free ends of barriers that are not connected to guardrail or concrete barrier, install a Type 3 Object Marker on the end face per Specification 705.
- 6. BARRIER DELINEATORS: Install Barrier Delineators in accordance with Specification Section 705. Mount the delineators on the top face of the barrier, with the roadway side of the delineator located 2" from the front face of the barrier and the reflective sheeting facing traffic of the nearest approach.
- 7. CRACK CONTROL: Provide ½" depth crack control V-Grooves at 15' to 30' spacing. Locate V-Grooves above any joint or discontinuity in the barrier footing. Align V-Grooves perpendicular to the longitudinal axis of the Pier Protection Barrier and make continuous across the top surface and both side faces. For slip formed barriers, score ½" V-Grooves while the concrete is still plastic, otherwise pre-form the joints when stationary forms are utilized.

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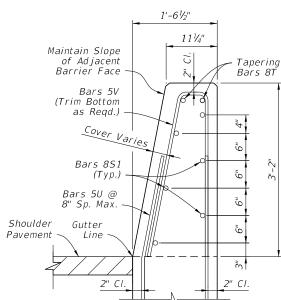


(Details Not Shown Below Gutter Line, See Sheet 6 for Footing and Stem Details) (Only Top & Bottom Longitudinal Steel Shown, See Section Views for All Steel Locations)

NOTES:

DESCRIPTION:

- 1. GENERAL: Construct either the 56" PPB or the 44" PPB height as called for in the Plans. See Sheets 2 & 3 for additional plan and elevation details.
- 2. FOOTING OPTIONS: See Sheet 6 for the supporting stem and footing details.



END VIEW C-C (Connects to Adjacent Concrete Barrier, Aligned at Gutter Line)

BARRIER DETAILS - CONNECTION TO CONCRETE BARRIER

LAST REVISION 11/01/17

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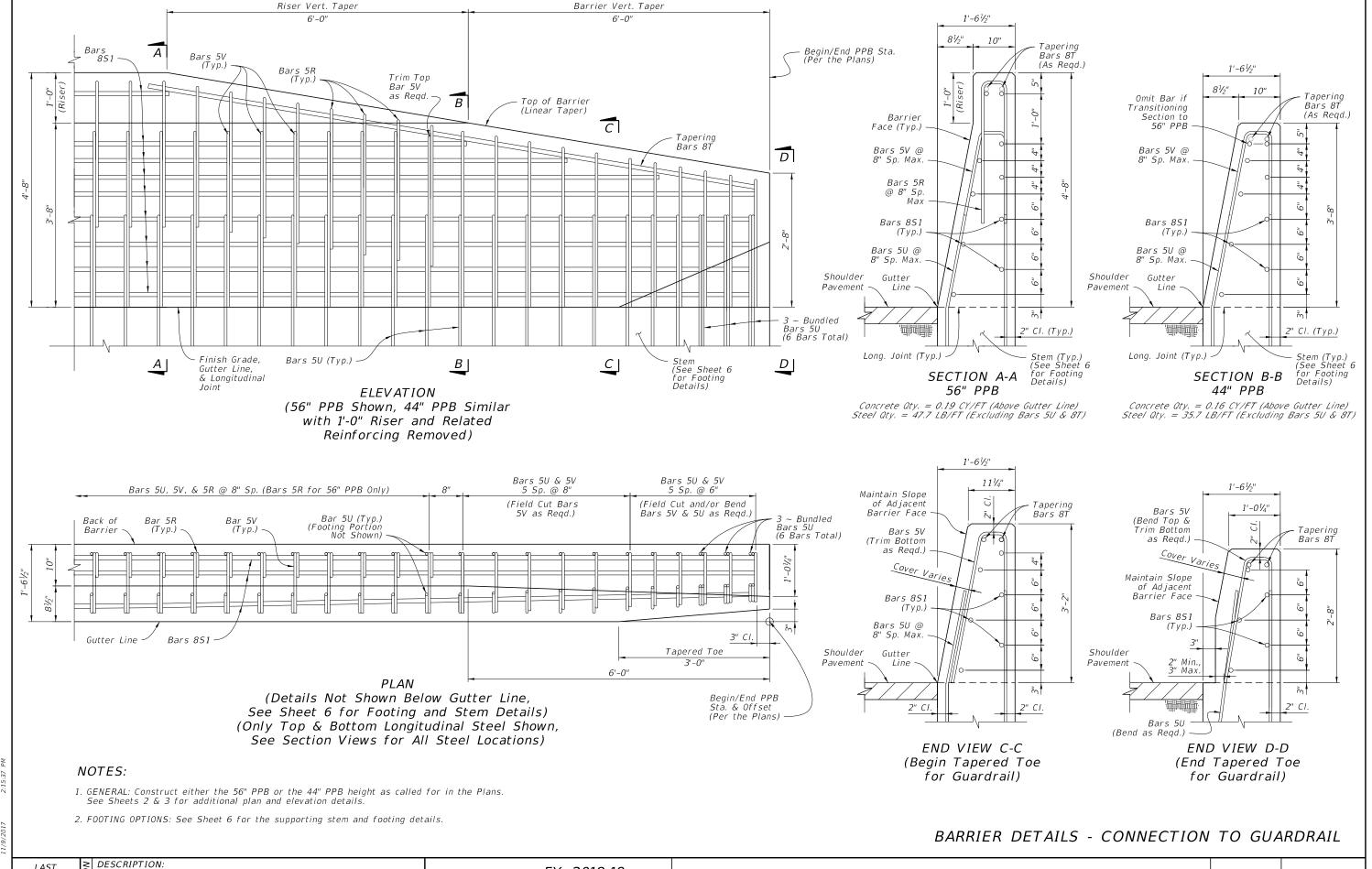
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PIER PROTECTION BARRIER

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SHEET



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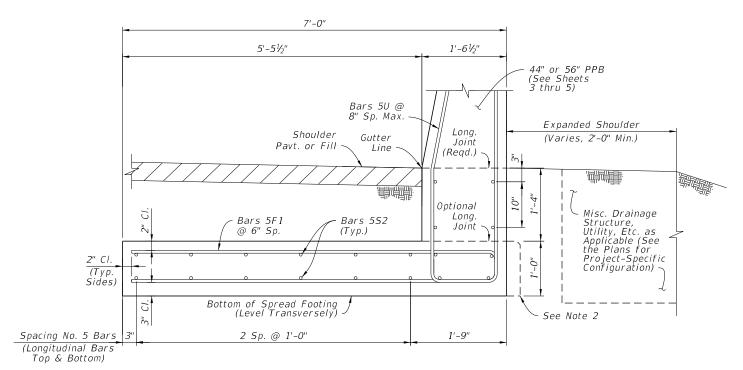
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FY 2018-19 STANDARD PLANS

PIER PROTECTION BARRIER

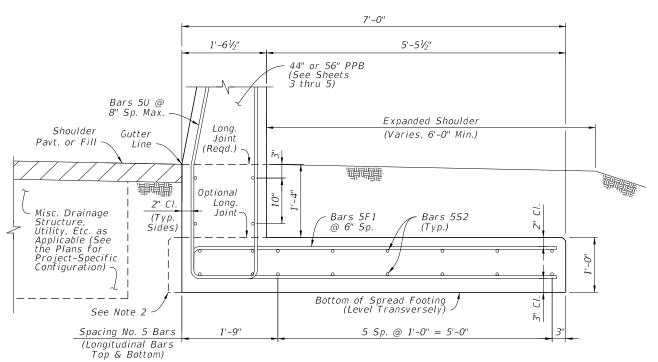
521-002

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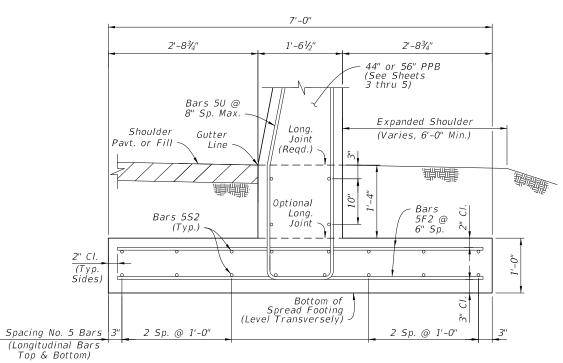
SECTION E-E FRONT-FLUSH FOOTING OPTION

Concrete Qty. = 0.34 CY/FT (Below Gutter Line) Steel Qty. = 63.5 LB/FT (Including Bars 50)



SECTION G-G REAR-FLUSH FOOTING OPTION

Concrete Oty. = 0.34 CY/FT (Below Gutter Line) Steel Oty. = 63.5 LB/FT (Including Bars 5U)



SECTION F-F SYMMETRICAL FOOTING OPTION

Concrete Qty. = 0.34 CY/FT (Below Gutter Line) Steel Qty. = 62.6 LB/FT (Including Bars 50)

NOTES:

1. GENERAL: Install the footing options per project-specific requirements, as defined on Sheet 2 and specified per the Plans.

Work with the supported 44" PPB and 56" PPB as shown on Sheets 3, 4, & 5.

- 2. OPTIONAL SLIP FORMING SUPPORT: The 1'-0" depth spread footing may be extended by 3" laterally beyond the face of the stem to provide support for a subsequent slip forming operation above. Do not adjust the steel reinforcement location for the additional concrete.
- 3. GUARDRAIL CONNECTION TAPERED TOE: For tapering the barrier as shown on Sheet 5, View D-D, bend Bars U away from the stem face as required. For this case, the cover requirement is variable for one side of the stem (only at the tapered toe locations).

BARRIER FOOTING OPTIONS

LAST

DESCRIPTION:

FDOT

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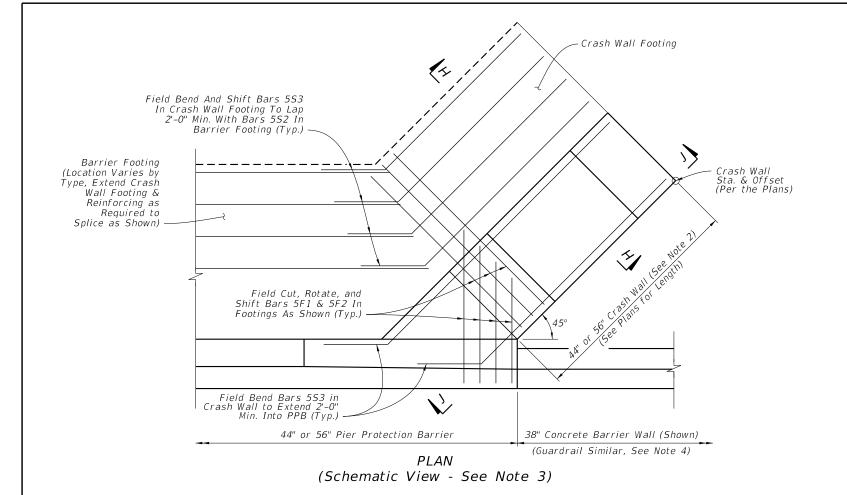
PIER PROTECTION BARRIER

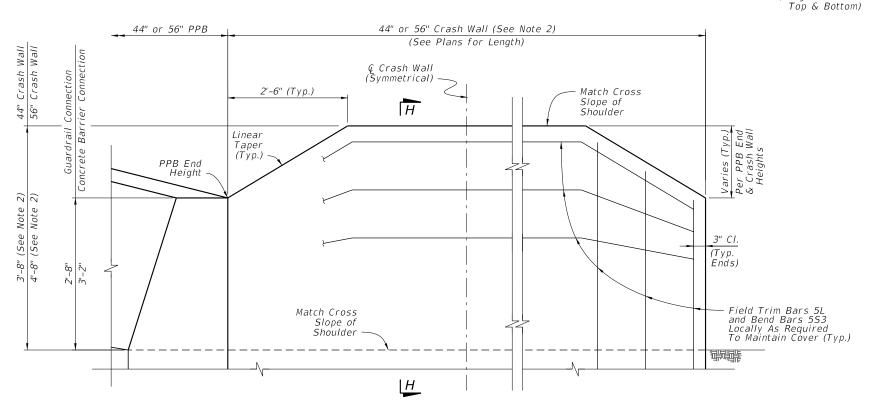
INDEX

SHEET

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7'-0" Crash Wall Crash Wall 4'-0" 3'-0" 44" 56" Bars 5E @ 1'-0" Sp. Max. (With Bars 5L) 553 (Typ., Wall & (Typ.)Stem) 3'-8" 1'-0" Bars 5L @ Bars 5L @ @ @ @ 1'-0" Sp. 1'-0" Sp. Sp. Sp. Max. Max. Long. Match Cross Joint Slope of Reqd. Shoulder Optional Bars 5F1 Bars 5S3 Joint @ 6" Sp. (Typ.)2" CI. Spacing Bars 5S3 (Typ. Sides) (Longitudinal Bars Each Face) Bottom of Spread Footing (Level Transversely) See Note 5 Spacing Bars 5S3 3 Sp. @ 1'-0" (Longitudinal Bars

Concrete Qty. = 0.82 CY/FT (44" Crash Wall) or 0.93 CY/FT (56" Crash Wall) Steel Qty. = 71.8 LB/FT (44" Crash Wall) or 76.0 LB/FT (56" Crash Wall)

SECTION H-H CRASH WALL

NOTES:

1. GENERAL: Only where called for in the Plans, install the Crash Wall as a supplement for PPB. If applicable, see the Plans for the corresponding Station and Offset required.

For additional layout details, see Sheets 2 & 3.

- 2. CRASH WALL HEIGHT: Install the Crash Wall at a height which matches the adjacent PPB (either 44" or 56").
- 3. SCHEMATIC VIEWS: Only partial reinforcing is shown in the Schematic Views to establish a trend while keeping clarity. For all reinforcing steel locations and spacing requirements, see Section H-H.
- 4. GUARDRAIL CONNECTIONS: To facilitate guardrail connections, shift the Crash Wall 3 feet from the end of the PPB as shown on Sheets 2 & 3.
- 5. OPTIONAL SLIP FORMING SUPPORT: The 1'-0" depth spread footing may be extended by 3" laterally beyond the face of the wall to provide support for a subsequent slip forming operation above. Do not adjust the steel reinforcement location for the additional concrete.

VIEW J-J CRASH WALL ELEVATION (Schematic View - See Note 3)

CRASH WALL DETAILS

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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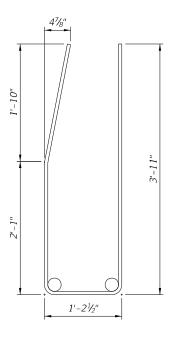
PIER PROTECTION BARRIER

INDEX

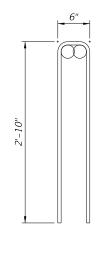
BILL OF REINFORCING STEEL				
MARK	SIZE LENGTH			
V	5	7'-5"		
U	5	8'-11"		
R	5	6'-0"		
F1	5	13'-9"		
F2	5	Varies (Straight)		
L	5	6'-5" / 7'-5"		
E	5	4'-6"		
S1	8	Varies (Straight)		
52, 53	5	Varies (Straight)		

	1'-21/2"
	81/8" 63/8"
3-6"	





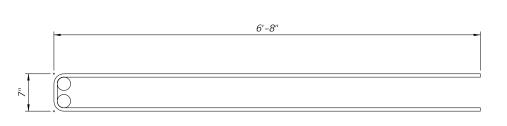
BARS 5U



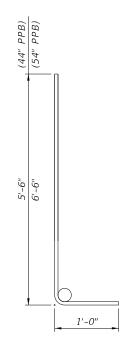
BARS 5R

NOTES:

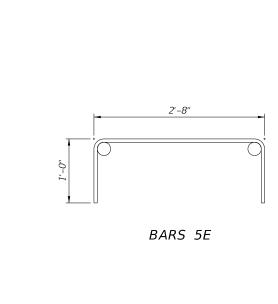
- 1. Work with the Standard Bar Bending Details per Index 415-001.
- 2. All bar dimensions in the bending diagrams are out to out.



BARS 5F1







BAR BENDING DIAGRAMS

≥ DESCRIPTION: REVISION 11/01/17

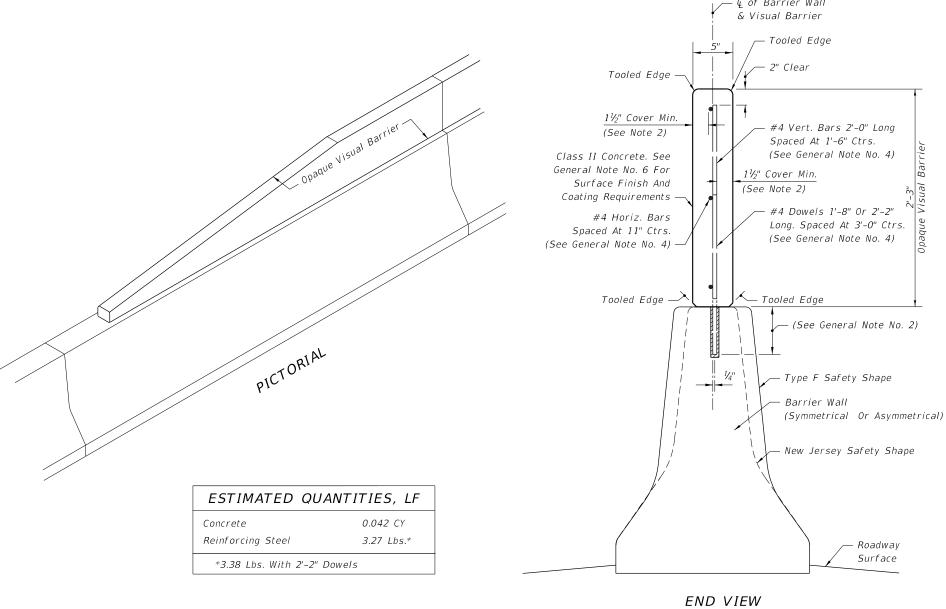
FDOT

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End Measurement For Opaque 1/3" Open Joint Visual Barrier Payment, LF Above Barrier Wall Joints Or Adjoining 6' Transition Rigid Structures 2" Clear Cut & Field Bend Reinf. Steel 2" Clear 2" Clear Top Of Concrete Barrier Wall ELEVATION OF REINFORCEMENT AND DOWELING © of Barrier Wall & Visual Barrier Tooled Edge



GENERAL NOTES

- 1. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when yielding to such strikes.
- 2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved non shrink grout. Embedment holes shall be 5#8" diameter, drilled to a depth 1#4" below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the 11#2" Cover Minimum to the face of the Opaque Visual Barrier.

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in the plans.

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

- 4.In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer.

 Details shall be submitted with requests for substitution.
- 5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineer's approval.

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations in Note No. 1 above.

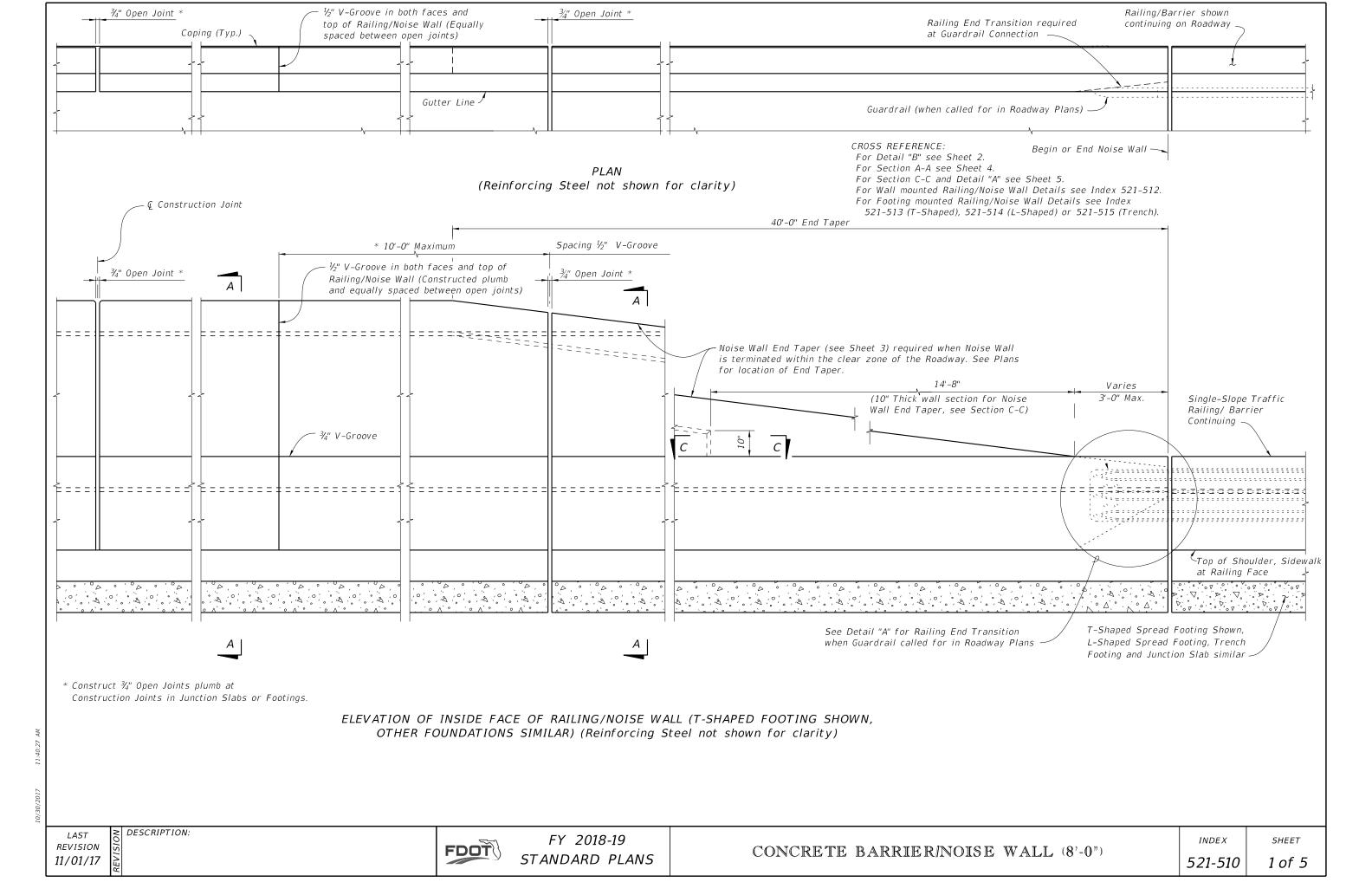
- 6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called for in the plans.
- 7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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Work this Index with Indexes 521-512 through 521-515.

CONSTRUCTION REQUIREMENTS: The Concrete Barrier/Noise Wall and joints shall be constructed plumb, they shall not be constructed perpendicular to the roadway surface.

CONCRETE: Class II for slightly aggressive environments and

Class IV for moderately or extremely aggressive environments.

BARRIER DELINEATORS: Install Barrier Delineators 2'-4" above the riding surface in accordance with Specification Section 705. Match the Barrier Delineators color (White or Yellow) to the near edgeline.

OPEN JOINTS: Provide 3/4" Open Joints spaced between 30 feet minimum or 90 feet maximum.

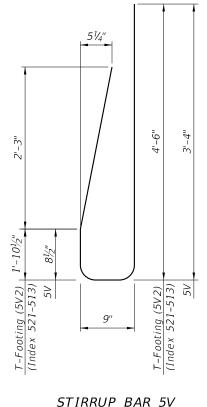
Align Open Joints with construction joints in the Junction Slab or Footing.

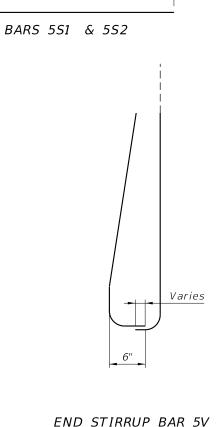
Provide additional reinforcing (see Sheet 3) at each open joint.

ESTIMATED TRAFFIC RAILING/NOISE WALL QUANTITIES					
ITEM UNIT QUANTITY					
Concrete (Railing) CY/LF 0.107					
Concrete (Noise Wall) CY/LF 0.136					
Reinforcing Steel (Typical) LB/LF 67.36					
Additional Reinf. @ Open Joint LB 262.58					

(The above quantities are based on the Concrete Barrier/ Noise wall typical section, (excluding junction slab or footing)

		REINI	ORCING	STEEL	BENDING	G DIAGRAMS
BILL OF	REINFORG	CING STEEL				
MARK	SIZE	LENGTH				
R1	5	5'-10"				
R2	5	7'-10"				
<i>S</i> 1	5	As Reqd.				
52	5	7'-3"				
V (Wall)	5	7'-1"				
V (T-Footing)	5	9'-5"			5S1 5S2	Length
5'-10"		5'-0½"	2'-3"	5½"	46"	BARS.





To Be Field Cut

(Railing End Transition)

Length as Required

7'-3"

REINFORCING STEEL NOTES:

(Field Cut and Bend for Railing End Transition)

BAR 5R2

BAR 5R1

1. All bar dimensions in the bending diagrams are out to out. 2. All reinforcing steel at the open joints shall have a 2" minimum cover.

3. Bars 5R shall be one continuous or lap spliced bar. No mechanical couplers are permitted.

4. Bars 5S1 may be continuous or spliced at the construction joints. Lap splices for Bars 5R and 5S1 shall be a minimum of 2'-2".

5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

6. See Index 521-514 and 521-515 for L-shaped and Trench footing vertical reinforcing.

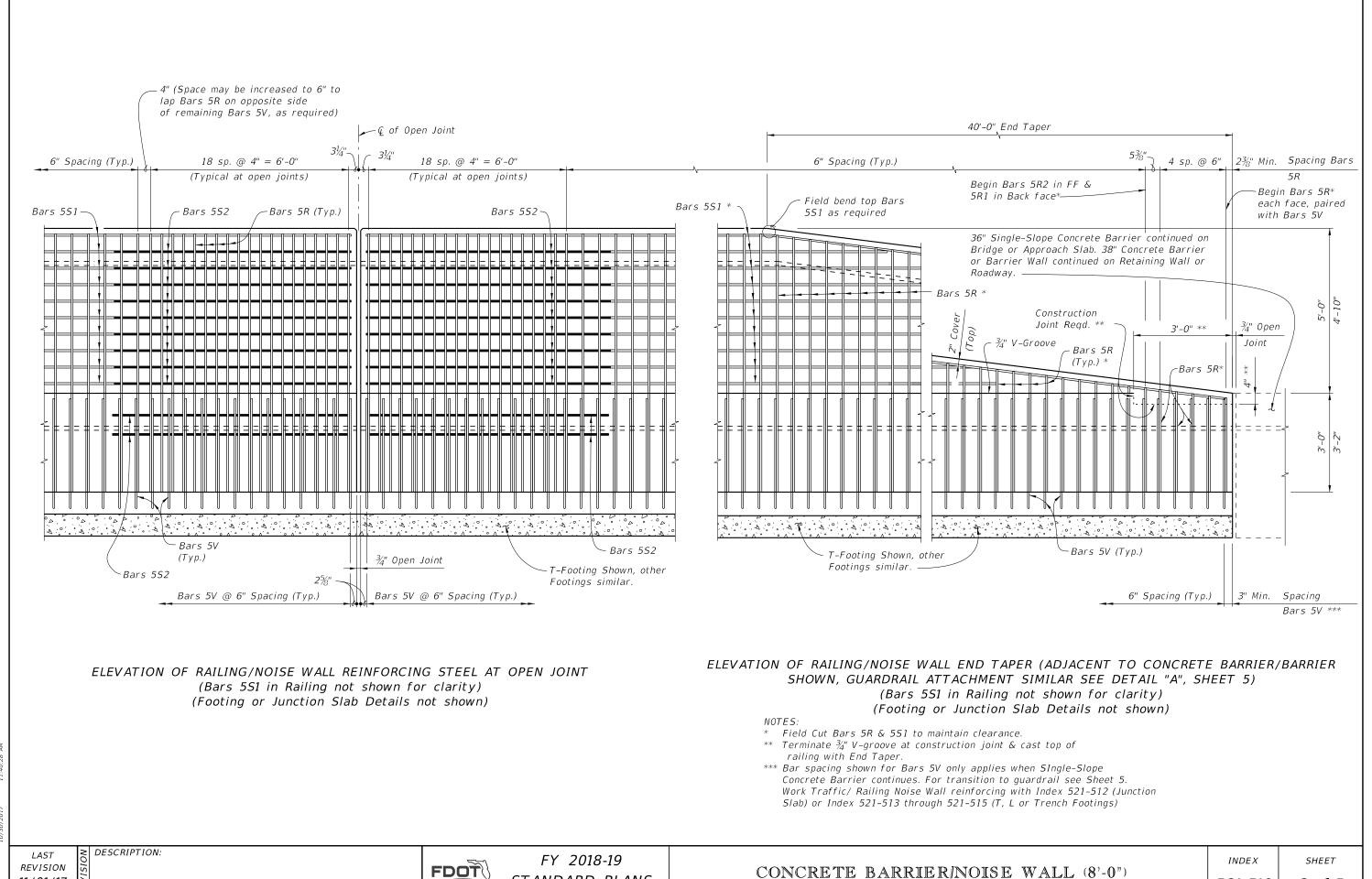
CROSS REFERENCE: See Index 521-512 for Junction Slab Details and Indexes 521-513 thru 521-515 for additional footing details.

REVISION 11/01/17

DESCRIPTION:

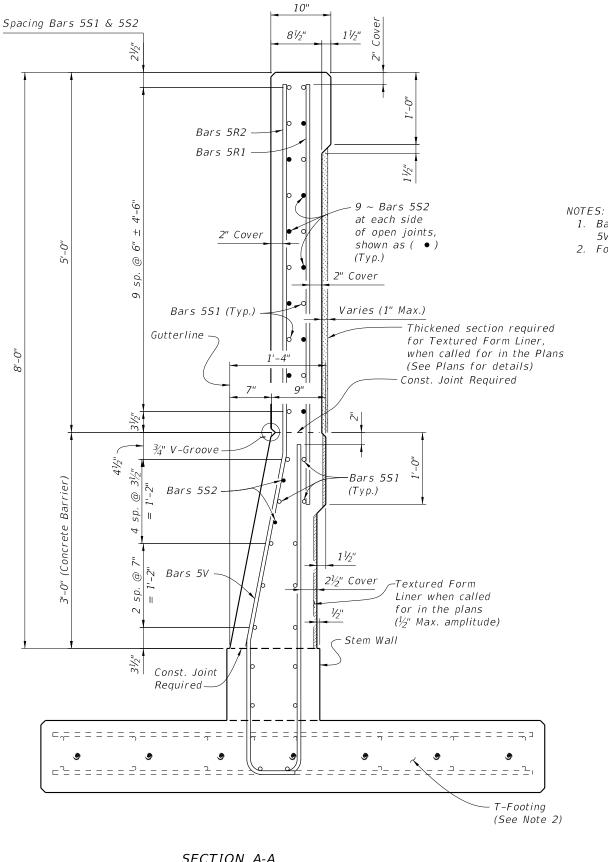
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STANDARD PLANS



SECTION A-A TYPICAL SECTION THRU CONCRETE BARRIER/NOISE WALL AT OPEN JOINT (Section Thru T-Footing Shown, Section Thru Junction Slab, L or Trench Footings similar)

CROSS REFERENCE:

For locations of Section A-A see Sheet 1. For location of View B-B, see Sheet 5. For Detail "A", see Sheet 5

1. Bars 5V shown are for T-Shape footings. 5V for Junction Slab, L-Shape and Trench footings are similar.

2. Foundation Details: Index 521-512 (Junction Slab) Index 521-513 (T-Shape) Index 521-514 (L-Shape) Index 521-515 (Trench)

> 1'-1" 31/4" 2" Cover (Top) ← Thrie Beam Terminal Connecter Bolts --Bars 5S1 (Field Bend as required to maintain cover (Typ.) Bars 5V (cut and lap as shown, (See Detail "A" for bar spacings) Riding Surface Const. Joint Required Edge of Stem Wall (See Note 2) Stem Wall Bars 5S (Field Bend as Regd.)

VIEW B-B END VIEW OF RAILILNG END TRANSITION FOR GUARDRAIL ATTACHMENT (T-Footing shown, Junction Slab, L or Trench Footings similar)

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DESCRIPTION:

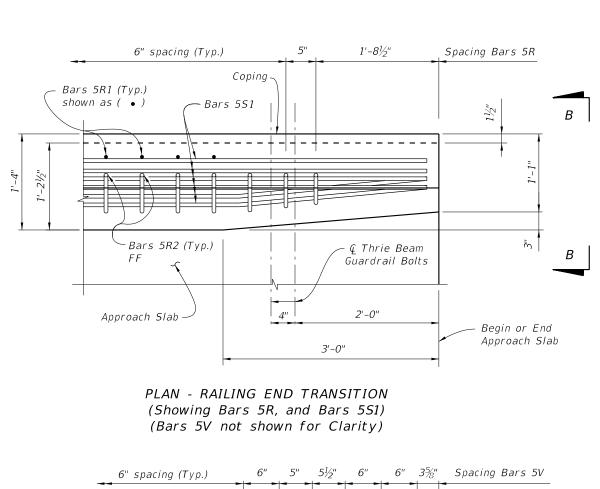
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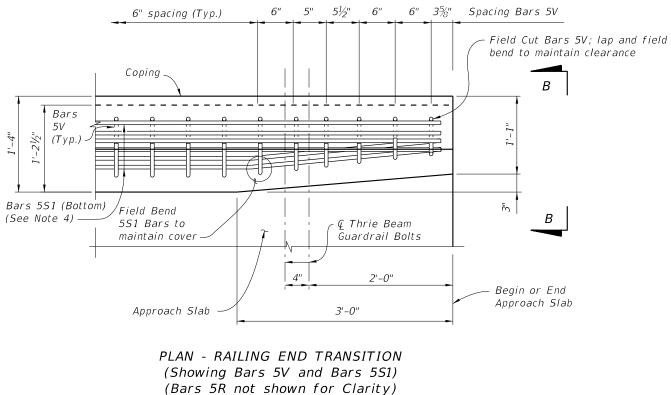
FY 2018-19 STANDARD PLANS

CONCRETE BARRIER/NOISE WALL (8'-0")

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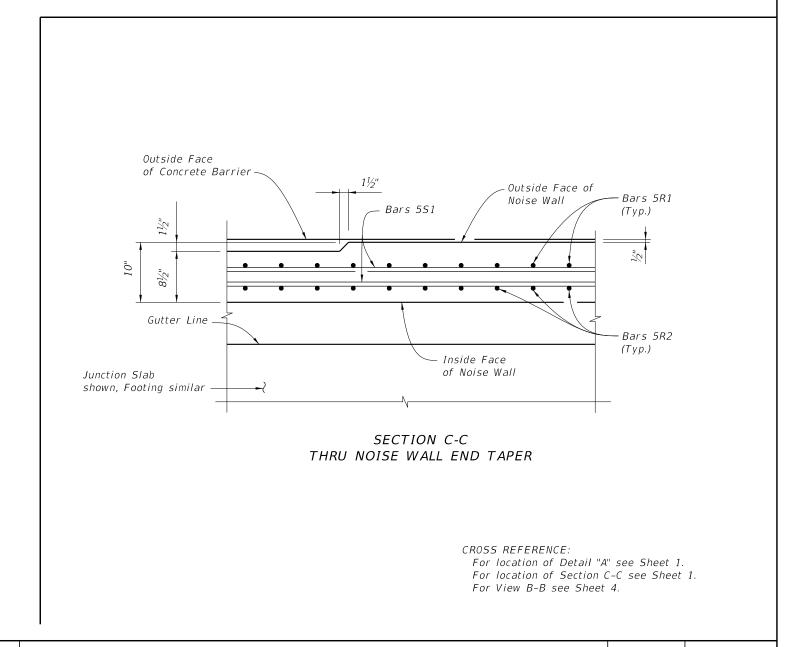




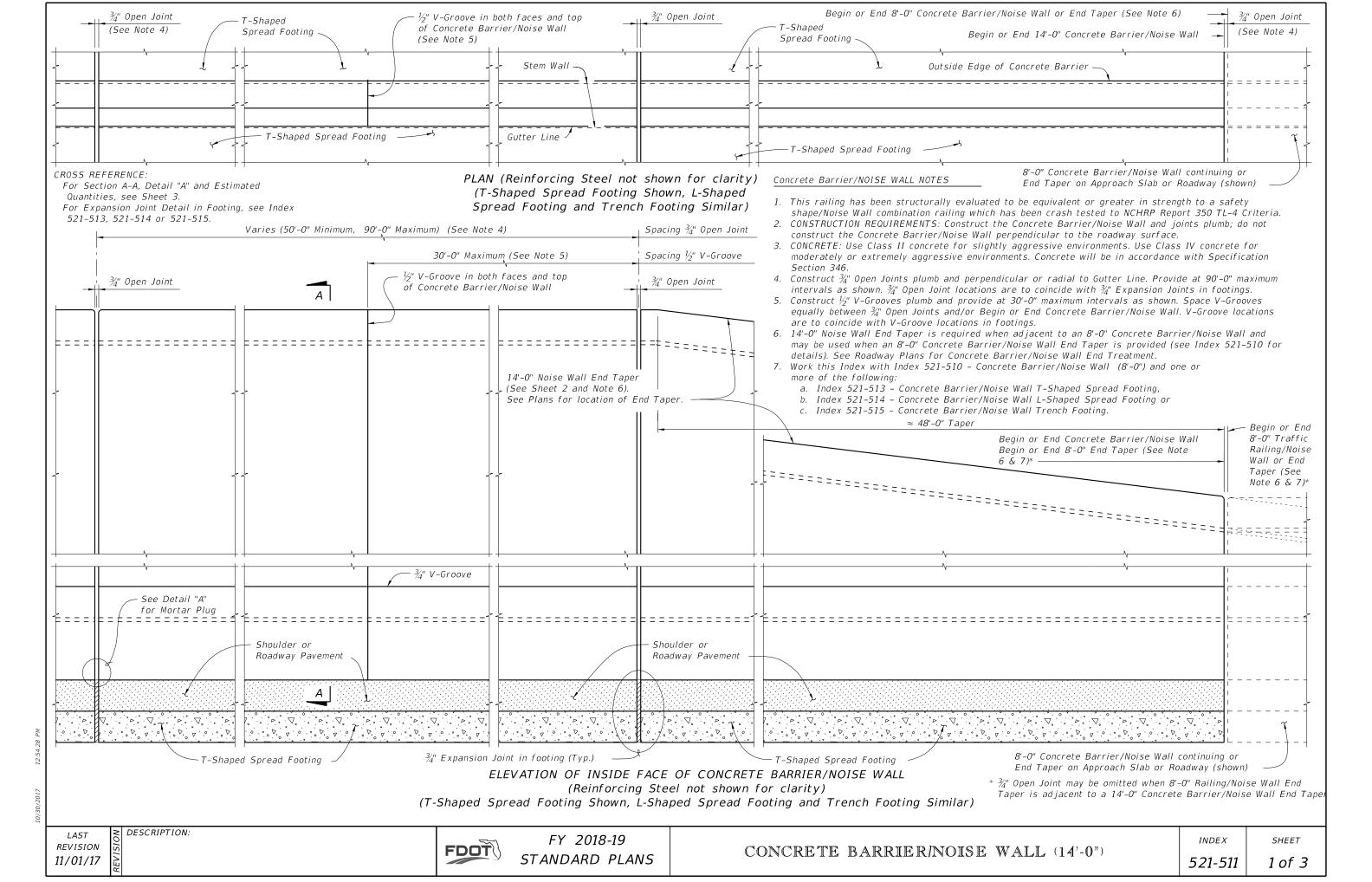
= DETAIL "A" ====

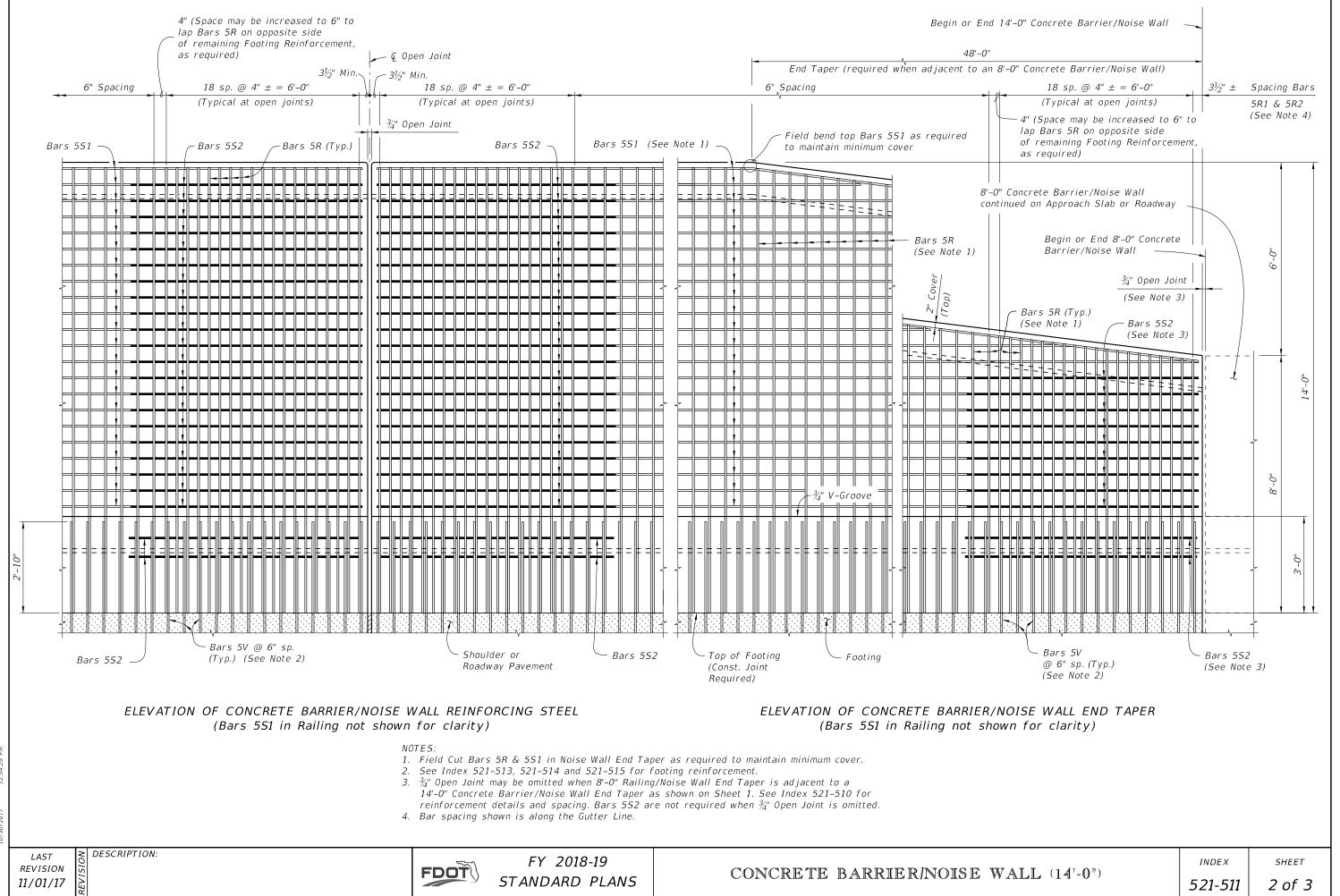
DETAIL "A" NOTES:

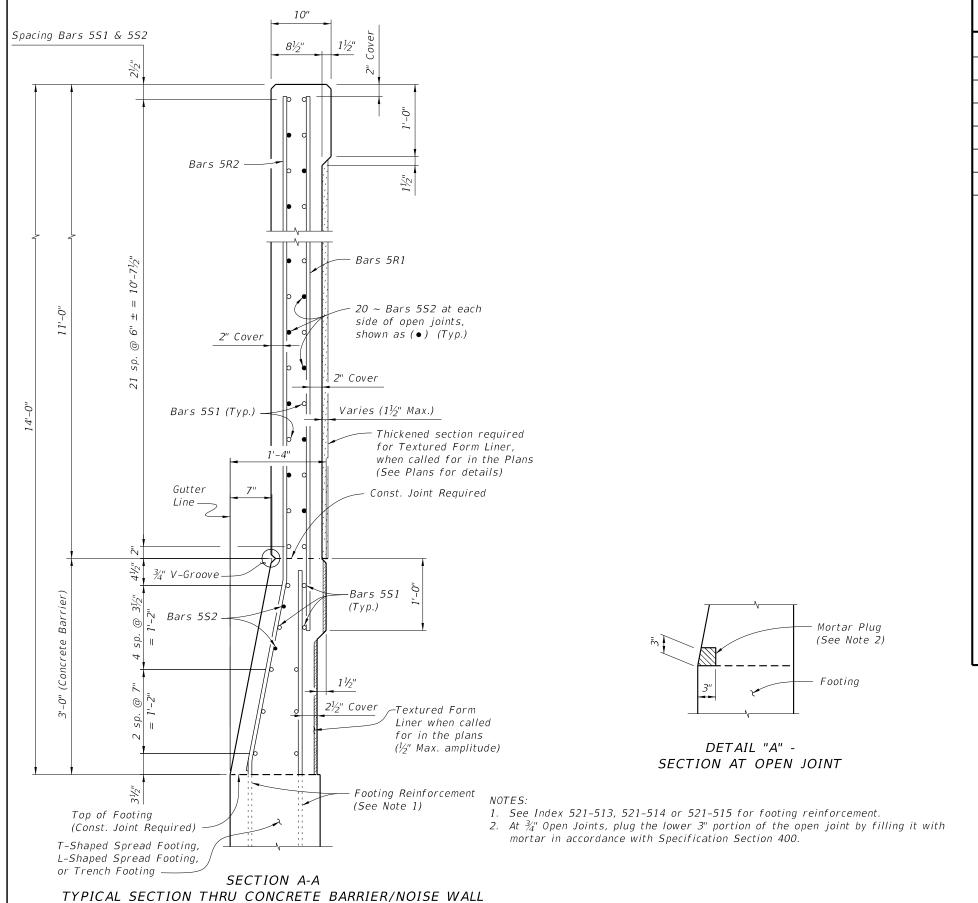
- 1. Begin placing Railing Bars 5V at the railing end and proceed toward the guardrail (thrie beam) terminal connector to ensure placement of guardrail bolt holes. Pair Bars 5R with Bars 5V as shown. Clearance of Bars 5R & 5V to guardrail bolt holes shall be checked to prevent cutting of bars if bolt holes are to be drilled. Shift bars locally where conflicts occur.
- 2. For Guardrail connection details see Index 536-001.
- . Omit Raililng End Transition if a Single-Slope Concrete Barrier/ Barrier continues beyond the End Taper. See the Plan Sheets.
- 4. Field cut Bars 5R1 to maintain cover. Field cut Bars 5V and lap as necessary to maintain cover; field cut & bend Bars 5R2 front leg (more plumb) to maintain cover and tie to S1 Bars. (See Sheet 4 Notes 1 and 2)



DESCRIPTION:

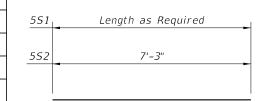




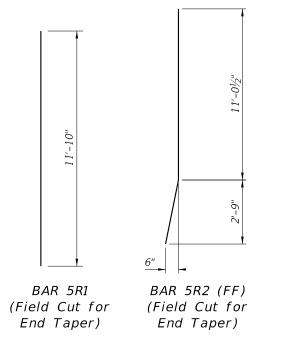


REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEE				
MARK	SIZE	LENGTH		
R1	5	11'-10"		
R2	5	13'-10"		
<i>S1</i>	5	AS REQD.		
52	5	7'-3"		
R2	5	2'-10"		



BARS 5S1 & 5S2



REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Bars 5R may be continuous or spliced at construction joints. Lap splices for Bars 5R and 5S1 will be a minimum of 2'-2''.
- 4. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

ESTIMATED CONCRETE BARRIER/NOISE WALL QUANTITIES

ITEM	UNIT	QUANTITY
Concrete (Concrete Barrier)	CY/FT	0.107
Concrete (Noise Wall, excluding any thickening)	CY/FT	0.293
Reinforcing Steel (Railing/Noise Wall) (Bars R1, R2, S1 & V)	LB/FT	105.95
Additional Reinf. @ Open Joint (Railing/Noise Wall)	LB	488.12

For locations of Section A-A and Detail "A", see Sheet 1.

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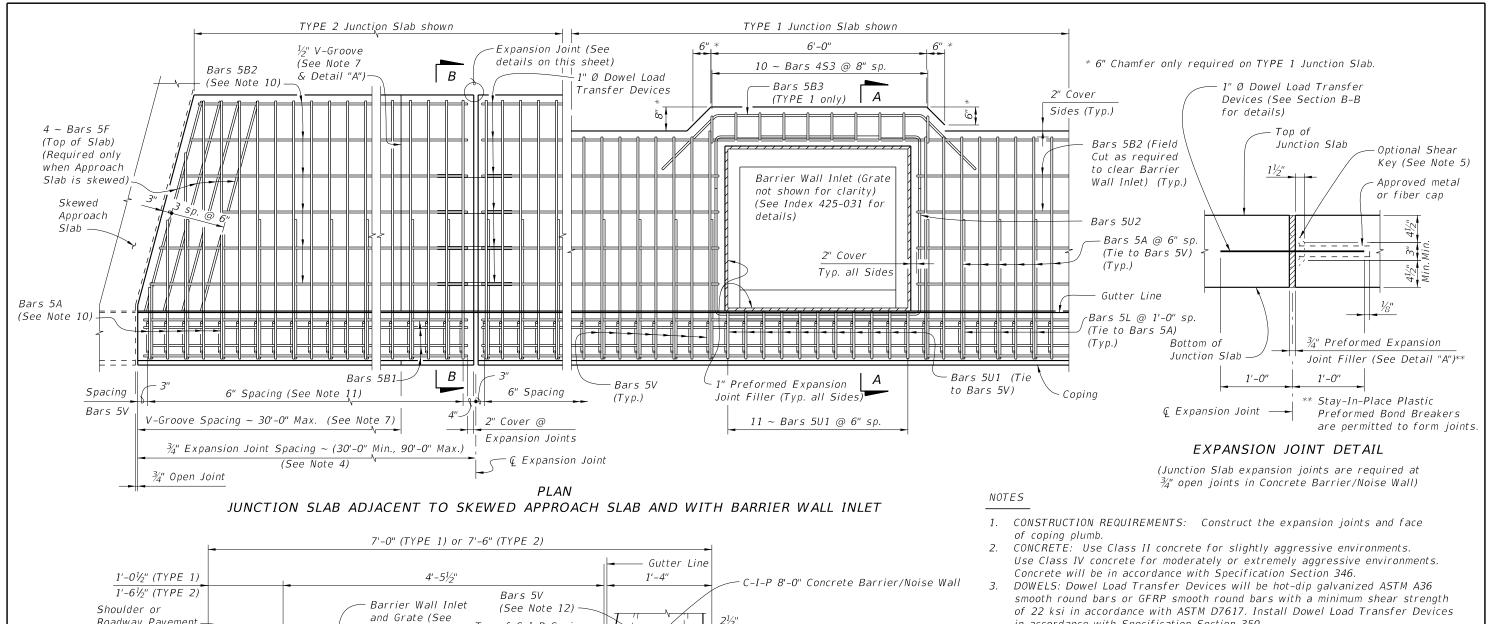
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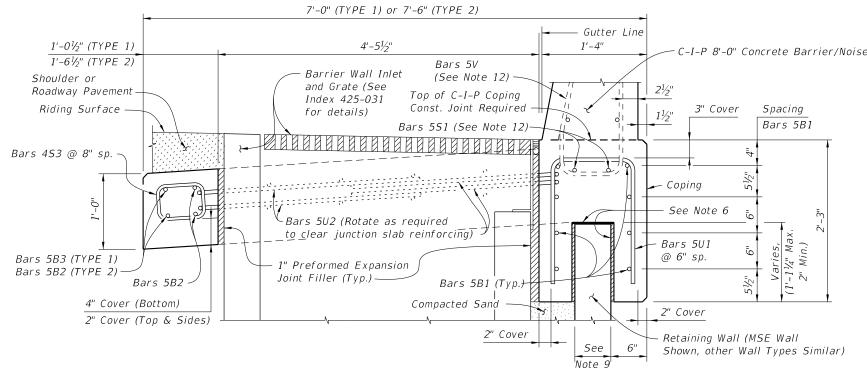
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CONCRETE BARRIER/NOISE WALL (14'-0")

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SECTION A-A SECTION THRU JUNCTION SLAB, BARRIER WALL INLET AND RETAINING WALL (TYPE 1 Junction Slab Shown, TYPE 2 Similar)

- in accordance with Specification Section 350.
- EXPANSION JOINTS: Construct $\frac{3}{4}$ " Expansion Joints plumb, and either perpendicular or radial to Gutter Line. Provide at 90'-0" maximum intervals as shown.
- Shear Keys in Junction Slab are required when GFRP bars are used for Dowel Transfer Devices and are optional with steel dowel bars. Tongue Slope on Shear Key must be constant and between 5° to 45° from horizontal.
- 6. Provide Organic Felt bond breaker on top and Expanded Polystyrene $(\frac{1}{5}" \text{ thick})$ on sides.
- 7. V-GROOVES: Construct $\frac{1}{2}$ " V-Grooves plumb and provide at 30'-0" maximum intervals as shown. Space V-Grooves equally between 3/4" Expansion Joints and/or Begin or End Junction Slab. V-Groove locations are to coincide with V-Groove locations in the Railing/Noise Wall.
- 8. FILL REQUIREMENTS: Shoulder or Roadway Pavement or Fill is required on top of the junction slab for its entire length on the traffic side of the Railing/Noise Wall. See Section B-B for details.
- Actual location & width vary depending on type of Retaining Wall used.
- 10. Field cut Bars 5A and 5B2 as required to maintain minimum cover for skewed Approach Slab.
- 11. Spacing shown is along the Gutter Line.
- 12. See Index 521-510 for Bars 5V and 2 ~ Bars 5S1. See Plans for Junction Slab width (TYPF).
- 13. Work this Index with Index 521-510 Concrete Barrier/Noise Wall (8'-0").

CROSS REFERENCE:

For Section B-B and Detail "A", see Sheet 2.

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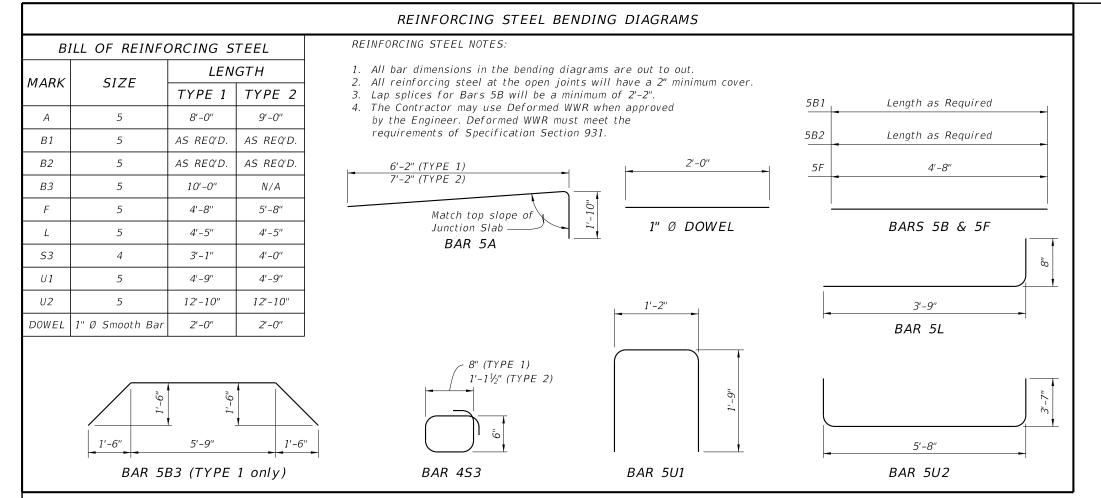
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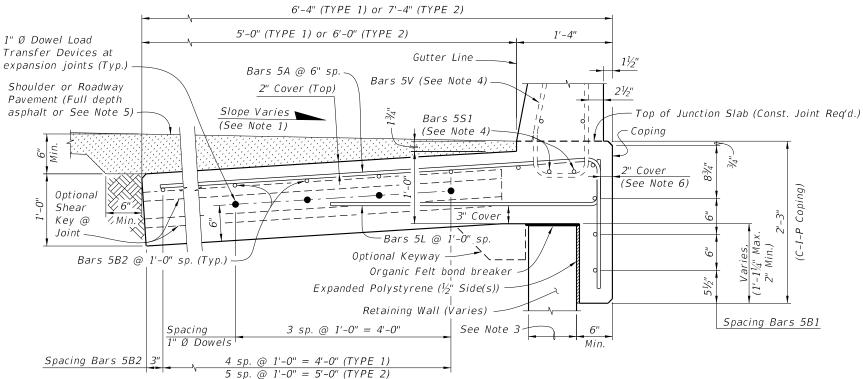
JUNCTION SLAB

INDEX

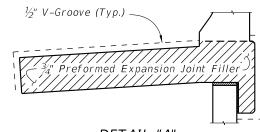
SHEET

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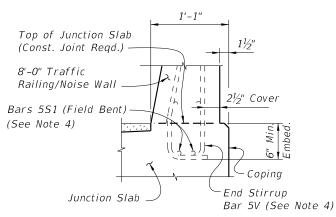


SECTION B-B TYPICAL SECTION THRU JUNCTION SLAB AND RETAINING WALL



DETAIL "A"

(Showing Locations of ½" V-Grooves and 3/4" Preformed Expansion Joint Filler)



PARTIAL END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V and Bars 5B1)

NOTE: See Index 521-510. Detail "A" for details.

ESTIMATED JUNCTION SLAB QUANTITIES						
QUANTITY						
ITEM UNIT TYPE 1 TYPE 2						
Concrete (Junction Slab) CY/FT 0.268 0.305						
Reinforcing Steel (Typical) LB/FT 31.72 34.85						
Additional Reinf. @ Expansion Joint	LB	21.36	21.36			

NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. Vary Junction Slab slope based on roadway cross slope to maintain a minimum 6" asphalt depth at the edge of the slab as shown.
- 3. Actual width varies depending on type of Retaining Wall used.
- 4. See Index 521-510 for Bars 5V and Bars 5S1.
- 5. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finished grade.
- 6. If slip forming is used, submit shop drawings for approval showing Expansion Joint support details and $2\frac{1}{2}$ " side cover with adjusted Typical Section dimensions.

CROSS REFERENCE:

For location of Section B-B, see Sheet 1.

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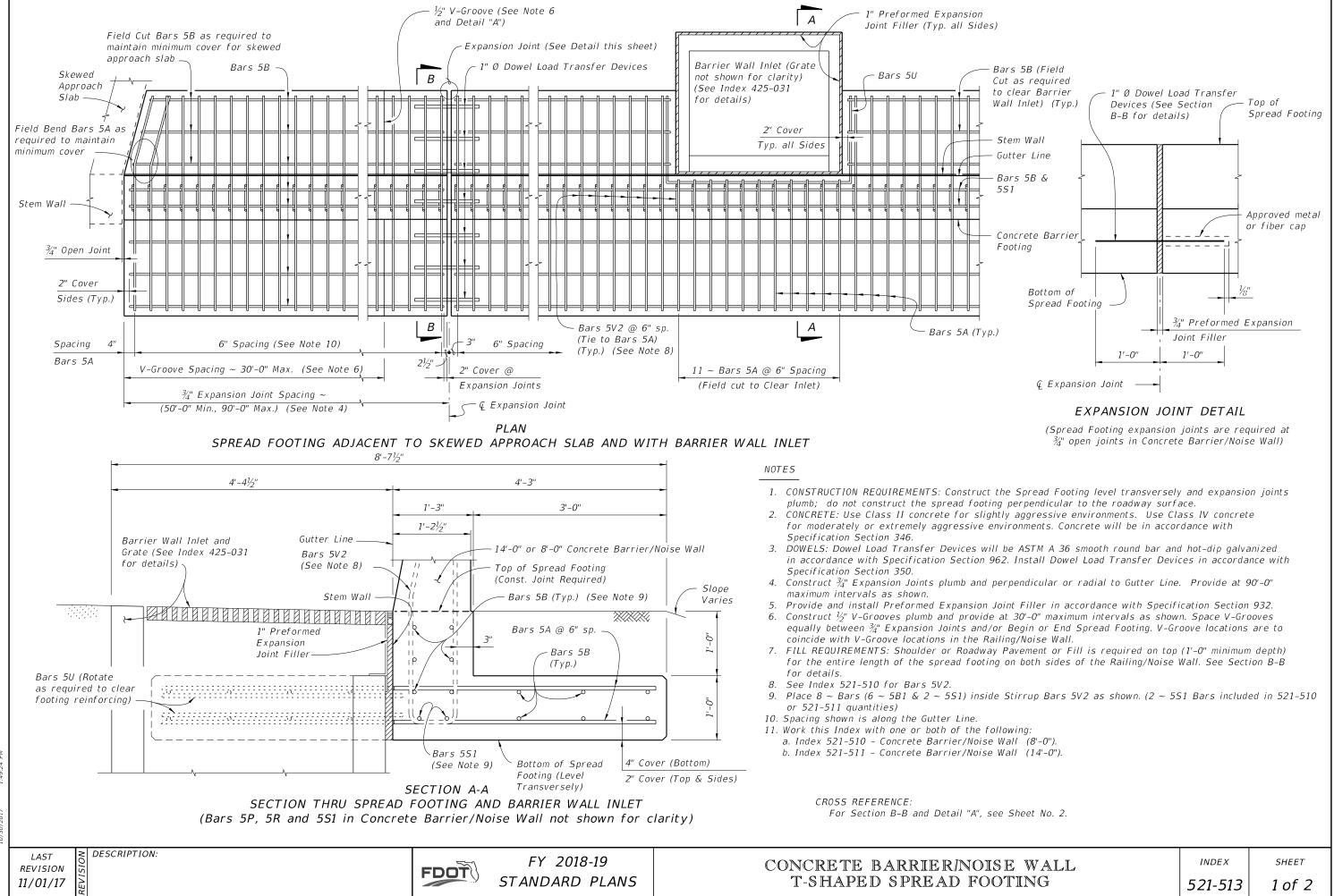
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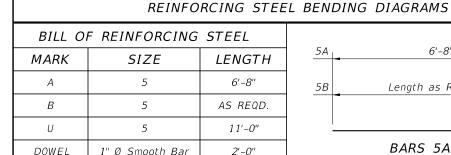
FY 2018-19 STANDARD PLANS

CONCRETE BARRIER/NOISE WALL (8'-0") JUNCTION SLAB

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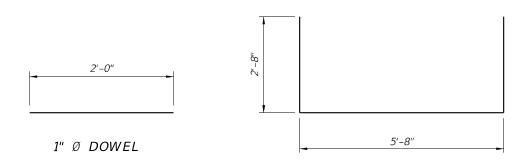


10/30/2017



5A -	6'-8"
5B	Length as Required

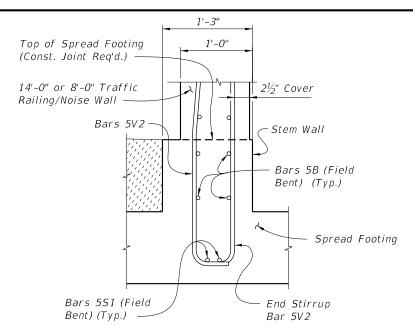
BARS 5A & 5B



BAR 5U

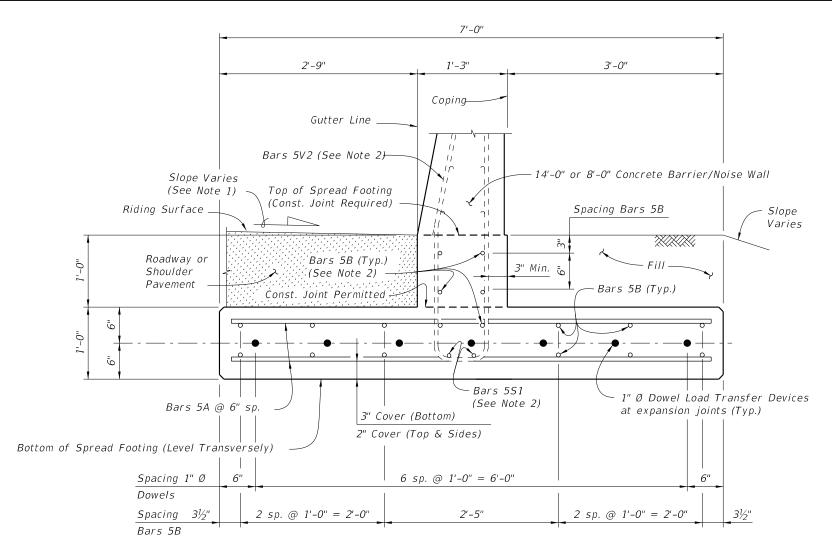
REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-2".
- 4. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.



PARTIAL END VIEW OF RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V2, and Bars 5B inside of Stirrup Bars 5V2)

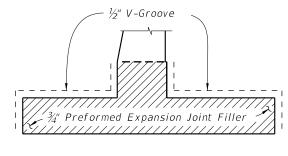
NOTE: See Index 521-510, Detail "A" for details.



SECTION B-B TYPICAL SECTION THRU SPREAD FOOTING (Bars 5R and 5S1 in Concrete Barrier/Noise Wall not shown for clarity)

NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. See Sheet 1, Notes 8 & 9.



DETAIL "A"

(Showing Locations of ½" V-Grooves and 3/4" Preformed Expansion Joint Filler)

ESTIMATED T-SHAPED SPREAD FOOTING QUANTITIES					
ITEM UNIT QUANTITY					
Concrete (Footing)	CY/FT	0.312			
Reinforcing Steel (Typical)	LB/FT	25.90			
Additional Reinf. @ Expansion Joint	LB	37.38			

Note: The reinforcing steel quantity includes the difference between Index 521-510 or 521-511 and Bars 5V shown. Bars 5S1 are included in Index 521-510 or 521-511 quantities.

CROSS REFERENCE:

For location of Section B-B, see Sheet 1.

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DESCRIPTION:

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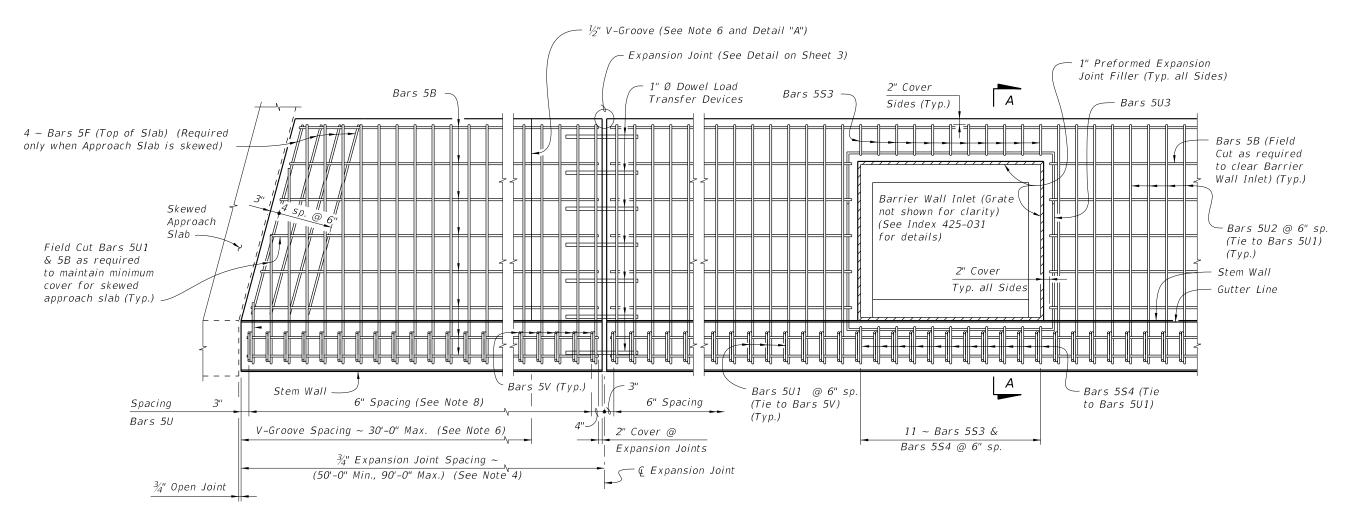
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CONCRETE BARRIER/NOISE WALL T-SHAPED SPREAD FOOTING

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PLAN - OPTION B SPREAD FOOTING ADJACENT TO SKEWED APPROACH SLAB AND WITH BARRIER WALL INLET (Option A Similar) (Bars S1 Not Shown)

NOTES

- 1. CONSTRUCTION REQUIREMENTS: Construct the Spread Footing level transversely and expansion joints plumb; do not construct the spread footing perpendicular to the roadway surface.
- 2. CONCRETE: Use Class II concrete for slightly aggressive environments. Use Class IV concrete for moderately or extremely aggressive environments. Concrete will be in accordance with Specification Section 346.
- 3. DOWELS: Dowel Load Transfer Devices will be ASTM A 36 smooth round bar and hot-dip galvanized in accordance with Specification Section 962. Install Dowel Load Transfer Devices in accordance with Specification Section 350.
- 4. Construct $\frac{3}{4}$ " Expansion Joints plumb and perpendicular or radial to Gutter Line. Provide at 90'-0" maximum intervals as shown.
- 5. Provide and install Preformed Expansion Joint Filler in accordance with Specification Section 932.
- 6. Construct ½" V-Grooves plumb and provide at 30'-0" maximum intervals as shown. Space V-Grooves equally between ¾" Expansion Joints and/or Begin or End Spread Footing. V-Groove locations are to coincide with V-Groove locations in the Concrete Barrier/Noise Wall.
- 7. FILL REQUIREMENTS: Shoulder or Roadway Pavement and Fill is required on the traffic side of the spread footing for a distance of 4'-0" and the full length of the spread footing (3'-0" minimum depth) on the backside of the spread footing for Option A. Fill is required for a distance of 4'-0" on the backside of the spread footing and the full length of the spread footing (3'-0" minimum depth) on the traffic side of the spread footing for Option B. See Typical Sections on Sheets 2 and 3 for details.
- 8. Spacing shown is along the Gutter Line.
- 9. Work this Index with one or both of the following:
 - a. Index 521-510 Concrete Barrier/Noise Wall (8'-0").
- b. Index 521-511 Concrete Barrier/Noise Wall (14'-0").

CROSS REFERENCE:

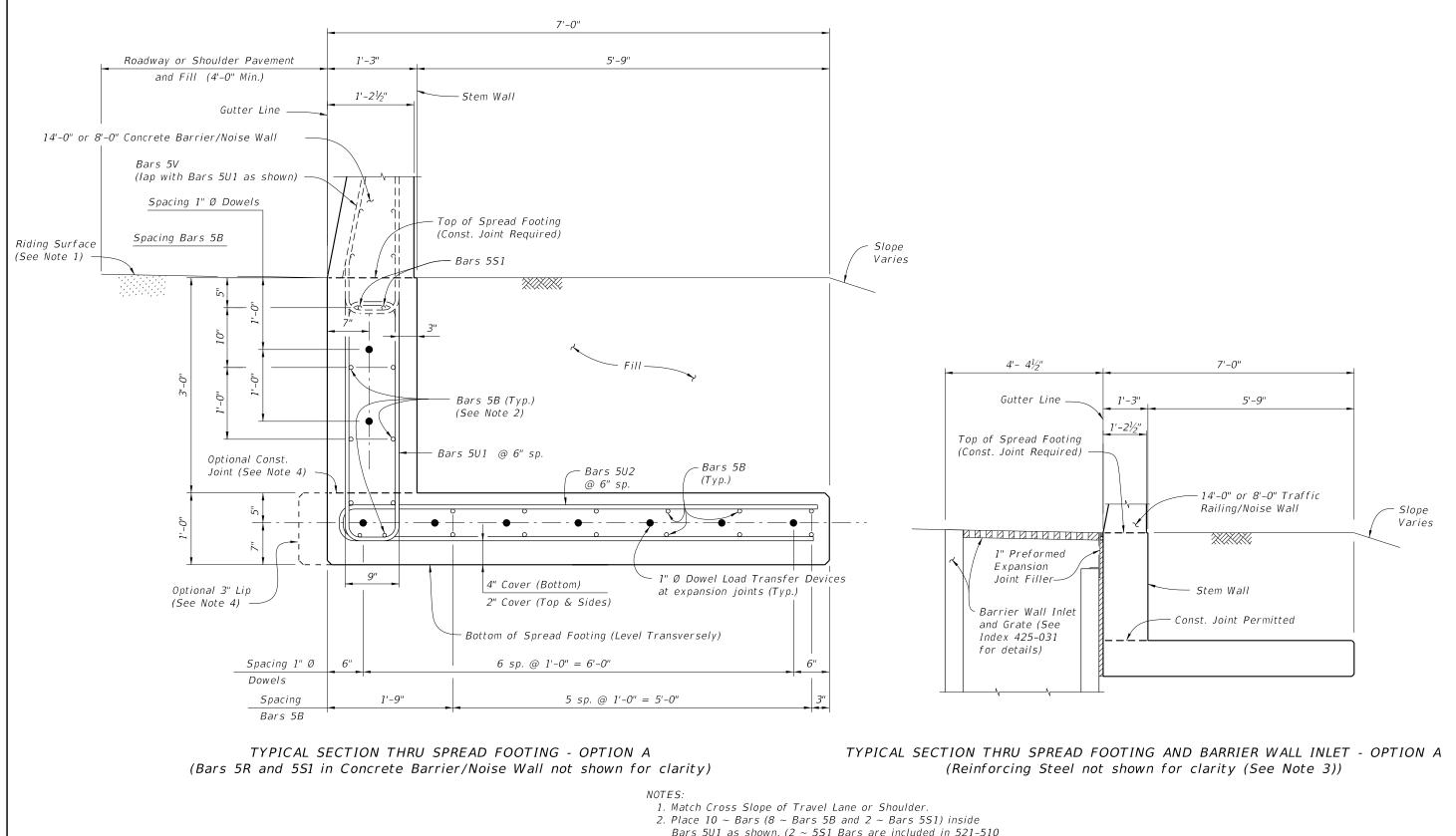
For Detail "A", see Sheet 3. For Section A-A and Estimated Quantities, see Sheet 4.

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DESCRIPTION:





- Bars 5U1 as shown, (2 ~ 5S1 Bars are included in 521-510 or 521-511 quantities)
- 3. For Reinforcing Steel spacing, see Typical Section Thru Spread Footing - Option A this Sheet.
- 4. Provide 3" lip when optional construction joint is used.

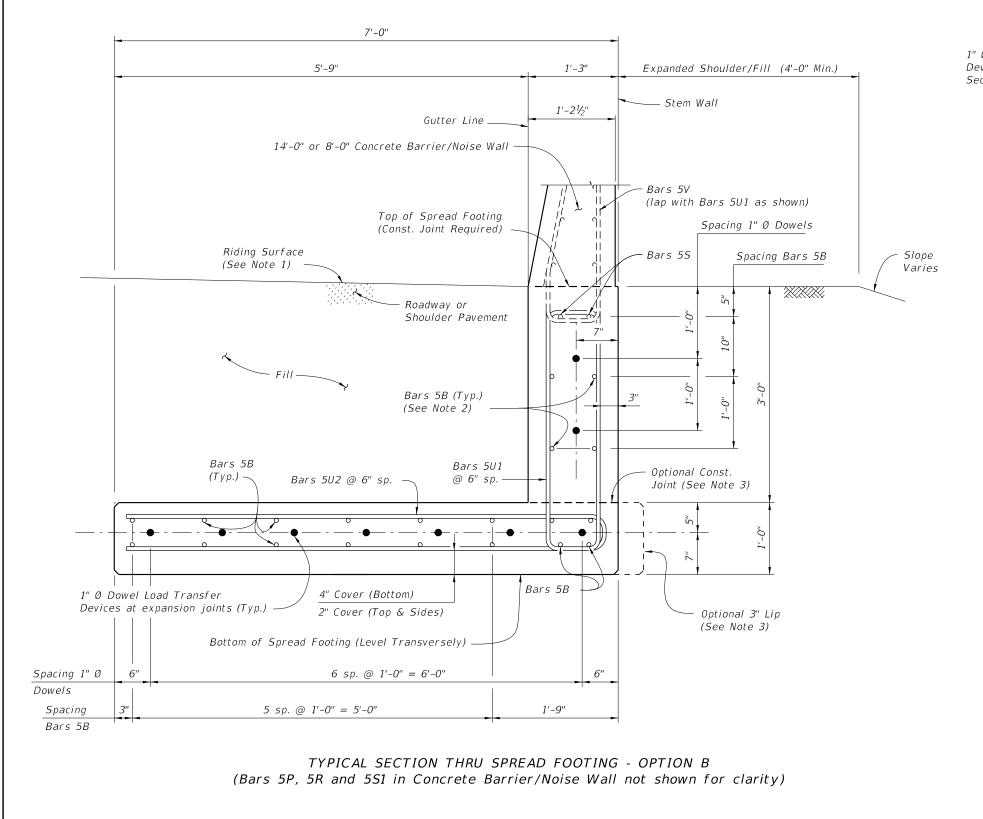
REVISION 11/01/17

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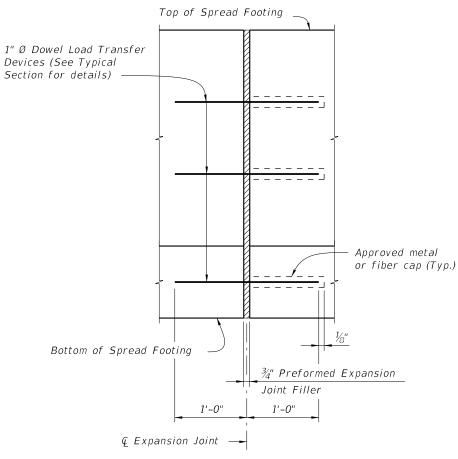
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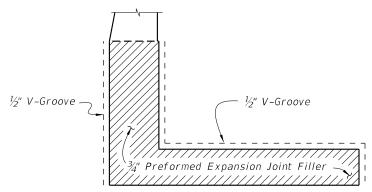
NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. Place 10 ~ Bars (8 ~ Bars 5B and 2 ~ Bars 5S1) inside Bars 5U1 as shown.
- 3. Provide 3" lip when optional construction joint is used.



EXPANSION JOINT DETAIL

(Spread Footing expansion joints are required at $\frac{3}{4}$ " open joints in Concrete Barrier/Noise Wall)



DETAIL "A" (Option A Shown, Option B Similar)

(Showing Locations of $\frac{1}{2}$ " V-Grooves and $\frac{3}{4}$ " Preformed Expansion Joint Filler)

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LAST REVISION 11/01/17

DESCRIPTION:

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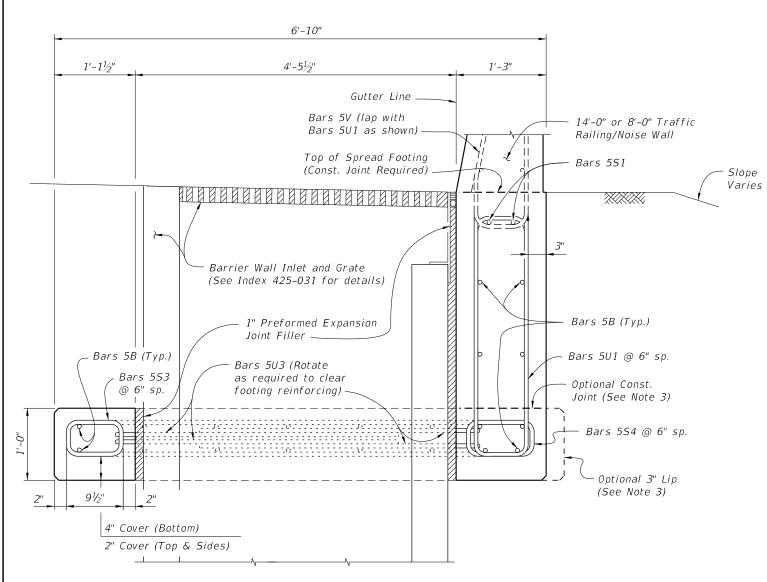
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CONCRETE BARRIER/NOISE WALL L-SHAPED SPREAD FOOTING

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SECTION A-A TYPICAL SECTION THRU SPREAD FOOTING AND BARRIER WALL INLET - OPTION B (Bars 5P, 5R and 5S1 in Concrete Barrier/Noise Wall not shown for clarity)

- 1. Place 8 ~ Bars 5B and 2 Bars S1 inside Bars 5U1 as shown.
- 2. For Reinforcing Steel spacing, see Typical Section Thru Spread Footing - Option B on Sheet 3.
- 3. Provide 3" lip when optional construction joint is used.

ESTIMATED L-SHAPED SPREAD	FOOTING	QUANTITIES
ITEM	UNIT	QUANTITY
Concrete (Footing)	CY/FT	0.398
Reinforcing Steel (Typical) *	LB/FT	68.84
Additional Reinf. @ Expansion Joint	LB	48.06

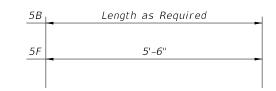
* Bars 5V and 5S1 are included in Index 521-510 or 521-511 quantiles.

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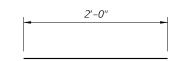
For location of Section A-A, see Sheet 1.

REINFORCING STEEL BENDING DIAGRAMS

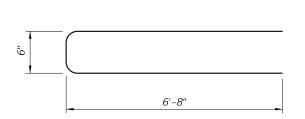
BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
В	5	AS REQD.		
F	5	5'-6"		
<i>S3</i>	5	3'-7"		
54	5	3'-10"		
U 1	5	9'-2"		
U2	5	13'-10"		
U3	5	12'-10"		
DOWEL	1" Ø Smooth Bar	2'-0"		

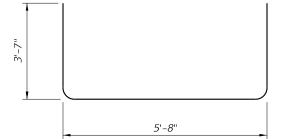


BARS 5B & 5F



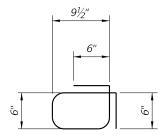
1" Ø DOWEL



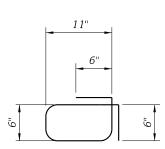


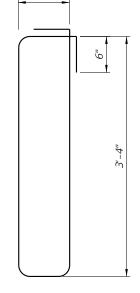
BAR 5U2

BAR 5U3



BAR 5S3





BAR 5S4 REINFORCING STEEL NOTES:

BAR 5U1

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-2".
- 4. Lap splices Bars 5T and 5V with 5U1 will be a minimum of 2'-2".
- 5. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

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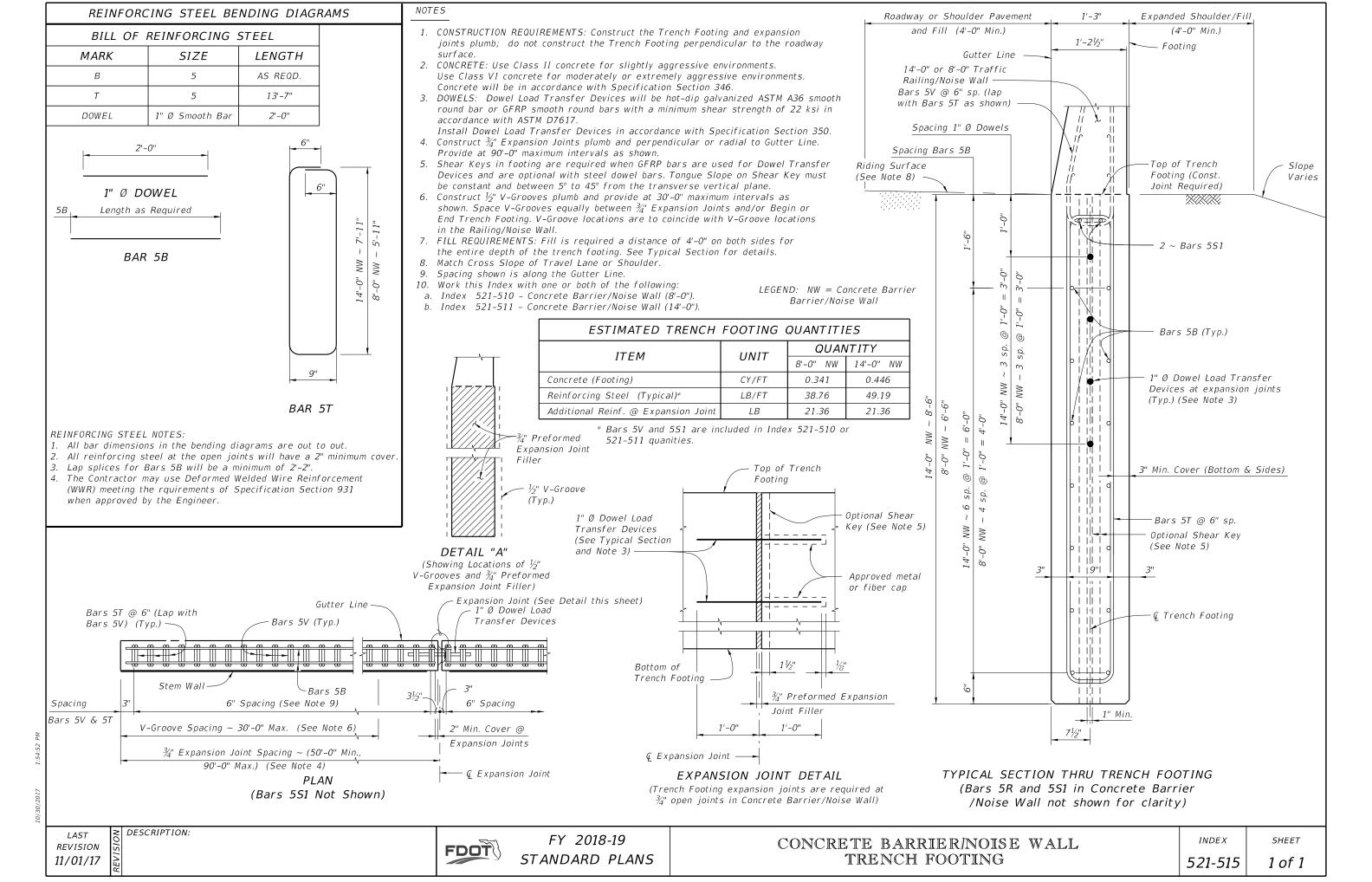
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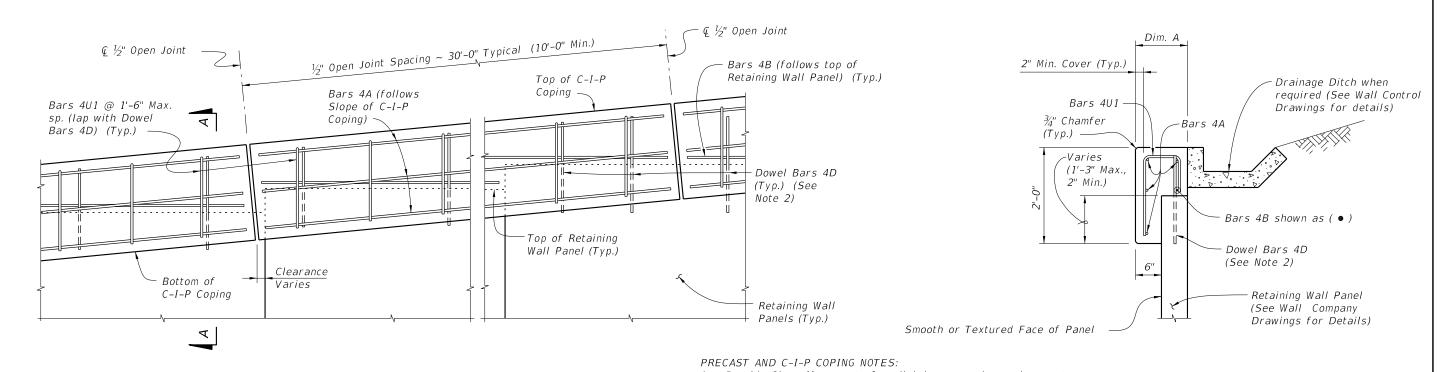
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CONCRETE BARRIER/NOISE WALL

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Ç ½" Open Joint

Top of Retaining

Wall Panel (Typ.)

C-I-P COPING - PARTIAL ELEVATION VIEW

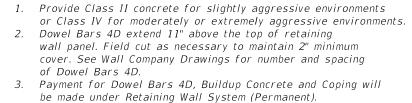
В

10'-0" Typical Precast Coping (5'-0" Min.)

Top of Precast Coping

Bottom of

Precast Coping



Retaining Wall

Panels (Typ.)

Dim. B 2" Min. Cover (Typ.) Drainage Ditch when required (See Wall Control Drawings Bars 4U2 - Bars 4A for details) (Typ.)¾" Chamfer (Typ.) − 2" Cover -C-I-P Buildup Concrete (1'-1" Max.) Provide $3\frac{1}{2}$ " x $3\frac{1}{2}$ " preservative treated timber blocking @ 5' Max. Spacing for gaps > 1" Varies (1'-3" Max., Dowel Bars 4D 2" Min.) (See Note 2) ¾" Std. $(\frac{1}{2}'')$ Min. ~ $2\frac{1}{2}''$ Max.) Min. Retaining Wall Panel (See Wall Company

SECTION A-A

C-I-P COPING

Smooth or Textured Face of Panel Drawings for Details)

PRECAST COPING - PARTIAL ELEVATION VIEW

SECTION B-B PRECAST COPING

REVISION 07/01/14

DESCRIPTION:

FDOT

Top of C-I-P

Dowel Bars 4D (Typ.)

(See Note 2)

Buildup Concrete -

Bars 4U2 @

1'-4" sp. (Typ.)

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Clearance

Varies

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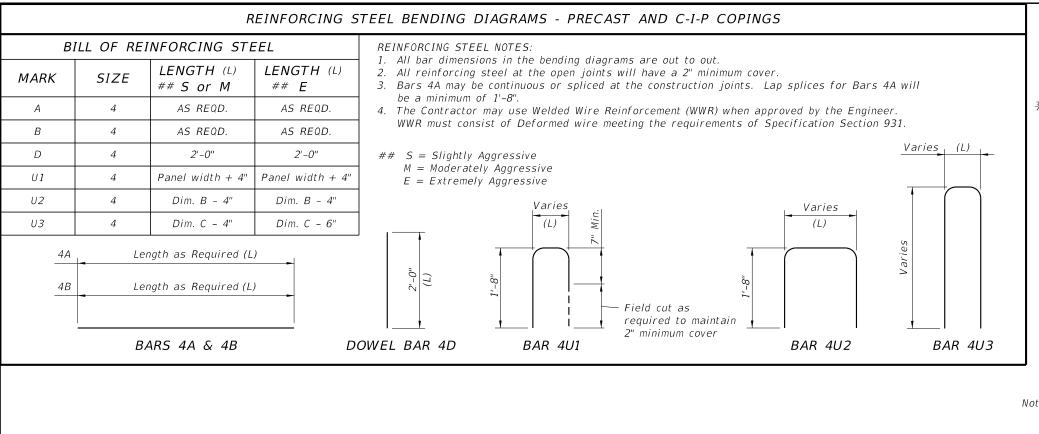
SHEET

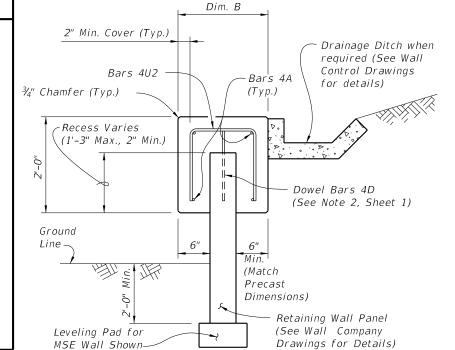
Panel width

+ 6"

Panel width

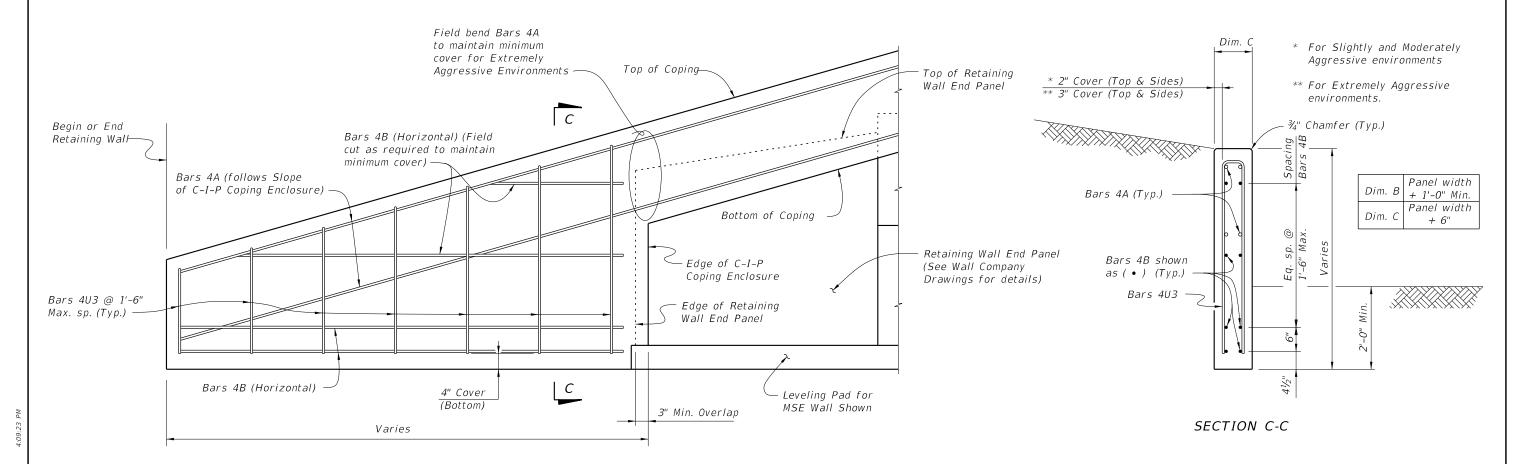
Dim. B + 1'-0" Min.





C-I-P COPING USED WITH PRECAST COPING

Note: When precast coping units do not fit the entire length of the retaining wall, use this similar C-I-P coping for short portions between precast coping units. This C-I-P coping may also be used for vertical copings.



C-I-P COPING ENCLOSURE DETAIL

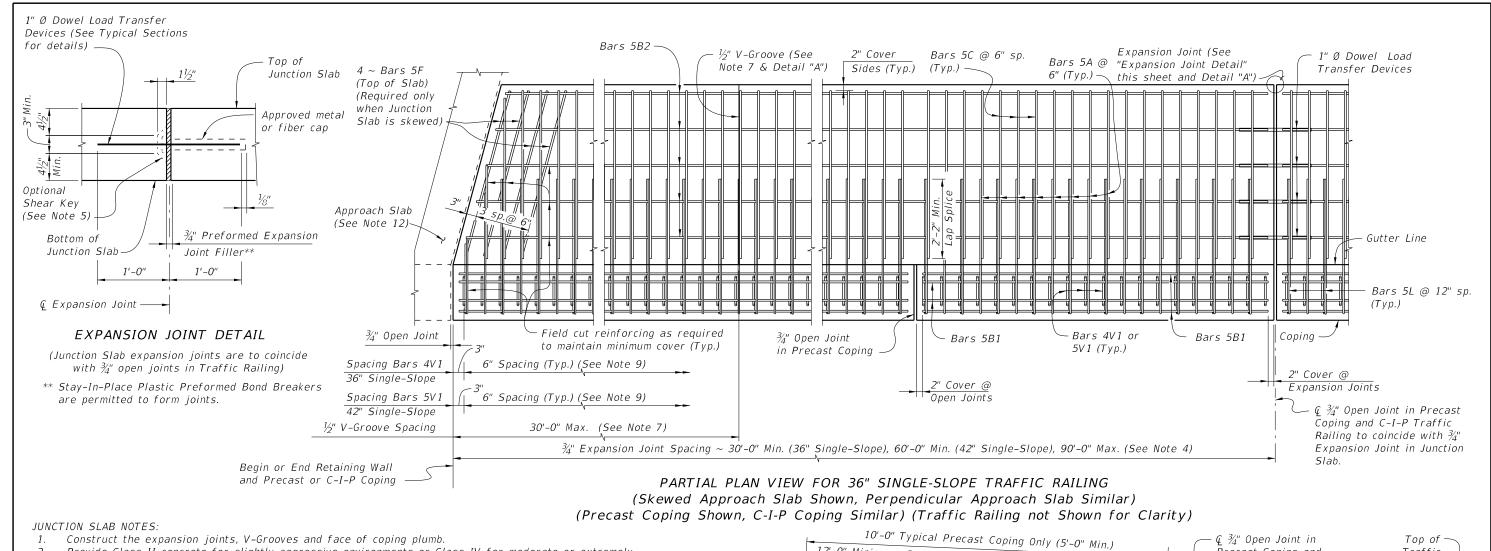
REVISION 07/01/13

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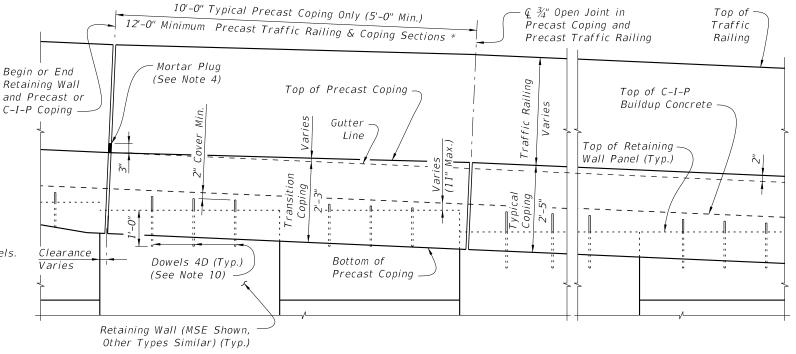
FDOT

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- Provide Class II concrete for slightly aggressive environments or Class IV for moderate or extremely aggressive environments.
- Dowel Load Transfer Devices will be hot-dip galvanized ASTM A 36 smooth round bar, or GFRP smooth round bars with a minimum shear strength of 22 ksi in accordance with ASTM D7617. Install Dowel Load Transfer Devices in accordance with Specification Section 350.
- Construct $\frac{3}{4}$ " Expansion Joints in junction slabs and C-I-P copings plumb and perpendicular or radial to the Gutter Line. Provide at 90'-0" maximum intervals as shown. Provide 3"x3" Mortar plugs in open joints at the base of traffic railings to contain runoff.
- Shear Keys in Junction Slab are required when GFRP bars are used for Dowel Transfer Devices and are optional with steel dowel bars. Tongue Slope on Shear Key must be constant and between 5° to 45° from horizontal
- Provide and install Preformed Expansion Joint Filler in accordance with Specification Section 932.
- Construct ½" V-Grooves in junction slabs and C-I-P copings at 30'-0" maximum intervals as shown. Space V-Grooves equally between 3/4" Expansion Joints and/or Begin or End Junction Slab. V-Groove locations are to coincide with V-Groove locations in the Traffic Railing.
- Shoulder or Roadway Pavement is required on top of the junction slab for its entire length on the traffic side of the Traffic Railing. See Typical Sections on Sheets 2 and 3 for details.
- Spacing shown is along the Gutter Line.
- For Precast Coping only, provide Dowel Bars 4D embedded 1'-0" and extended 9" above the top of MSE wall panels. Field cut as necessary to maintain 2" minimum cover to the top of the buildup concrete. See Wall Company Drawings for number and spacing of Dowel Bars 4D.
- The following Indexes contain details of the intersection of the retaining wall at approach slabs: Index 400-090 - Approach Slabs (Flexible Pavement Approaches) Index 400-091 - Approach Slabs (Rigid Pavement Approaches)



PARTIAL ELEVATION VIEW (Precast Coping and Junction Slab Reinforcing not Shown for Clarity) (Precast Coping Shown, C-I-P Coping Similar)

* C-I-P End Section must be ≥ 12'-0".

CROSS REFERENCE: For Detail "A", see Sheet 2.

REVISION 11/01/17

FDOT

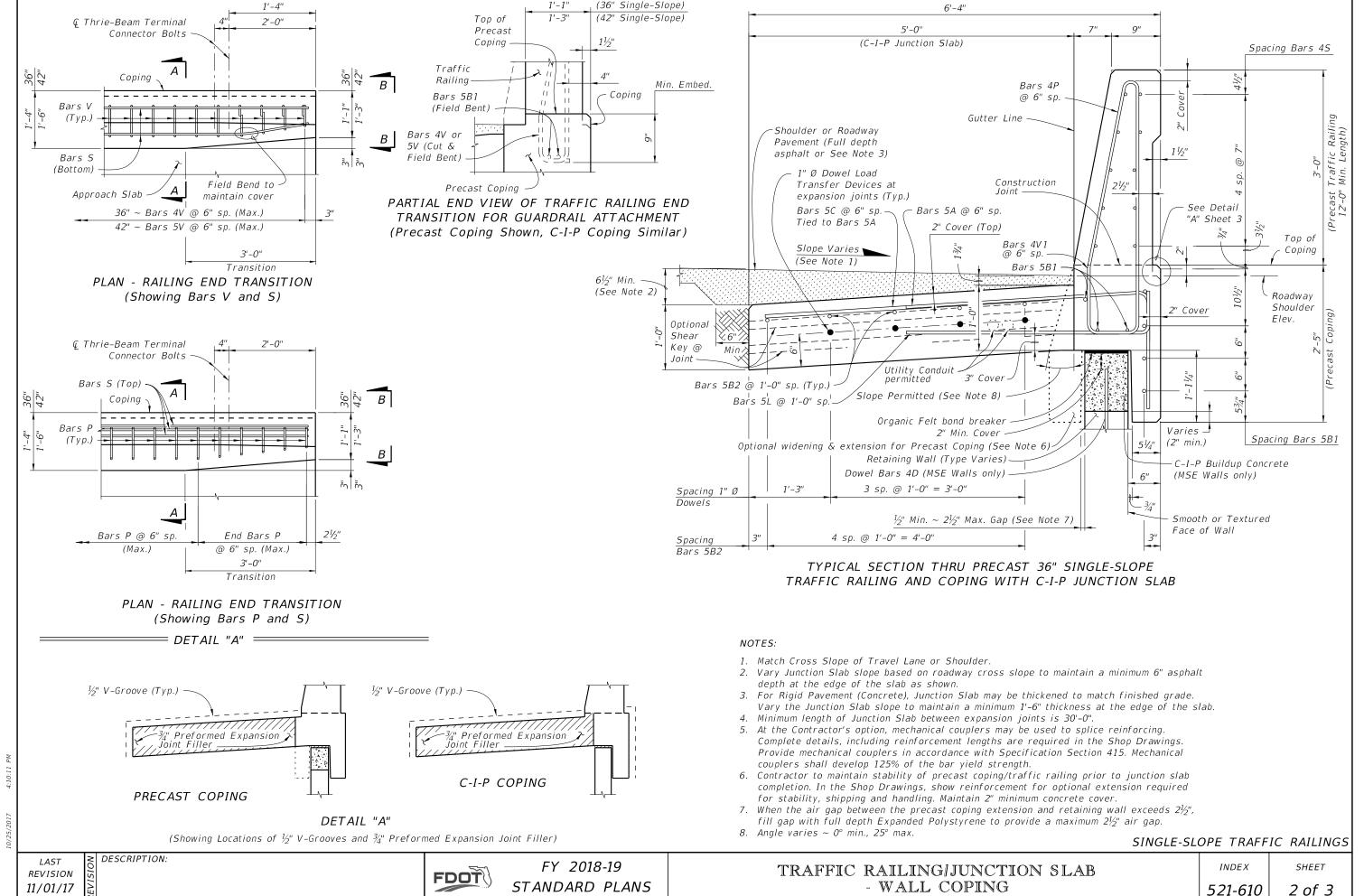
FY 2018-19 STANDARD PLANS

TRAFFIC RAILING/JUNCTION SLAB - WALL COPING

SINGLE-SLOPE TRAFFIC RAILINGS INDEX SHEET

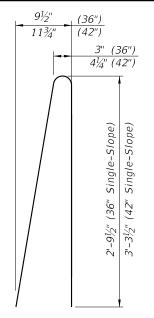
> *521-610* 1 of 3

DESCRIPTION:

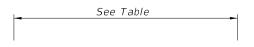


6'-4"

2'-0"



STIRRUP BAR 4P (36") 5P (42")



N/A

2'-0"

V 1

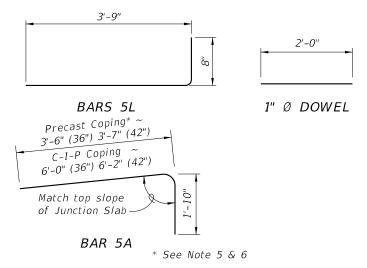
1" Ø

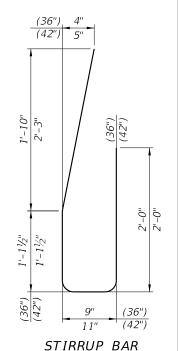
Dowel

5

Smooth

BARS 5B1, 5B2, 5C, 5F, 4S, & 6S





4V1 (36") 5V1 (42")

REINFORCING STEEL NOTES:

DESCRIPTION:

- 1. All bar dimensions in the bending diagrams are out to out.
- All reinforcing steel at expansion and open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B & 5S will be a minimum of 2'-2".
- 4. For Precast Copings only, lap splice Bars 5A with Bars 5C. Lap splices will be a minimum of 2'-2".

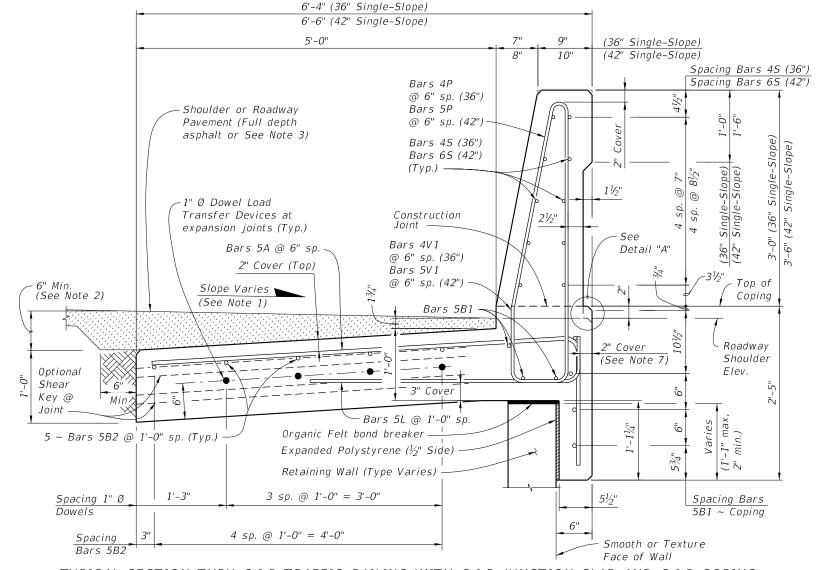
N/A

2'-0"

6'-4"

2'-0"

- 5. The Contractor may use either full length Bars 5A or lap splice with Bars 5C at Bars 5A for C-I-P Copings.
- 6. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 1'-2\frac{1}{2}'' (36" Single-Slope) or $1'-4\frac{1}{2}$ " (42" Single-Slope).
- 7. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 4'-8".
- 8. When approved by the Engineer, the Contractor may use deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931.
- 9. Contractor may use a single #5 stirrup in lieu of two bars for 4P and 4V1.

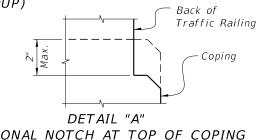


TYPICAL SECTION THRU C-I-P TRAFFIC RAILING WITH C-I-P JUNCTION SLAB AND C-I-P COPING (PRECAST COPING SIMILAR WITH C-I-P BUILDUP)

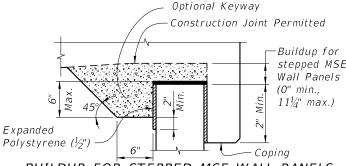
- 1. Match Cross Slope of Travel Lane or Shoulder
- 2. Vary the Junction Slab slope based on the roadway cross slope to maintain a minimum 6" asphalt depth at the edge of the slab.
- 3. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finish grade. Vary the Junction Slab slope to maintain a minimum 1'-6" thickness at the inside edge of the slab.
- 4. Minimum length of Junction Slab between expansion joints is 30'-0" for 36" Single-Slope or 60'-0" for 42" Single-Slope.
- Contractor to maintain stability of precast coping prior to junction slab completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 2" minimum concrete cover.
- 6. If slip forming is used, submit shop drawings for approval showing $2\frac{1}{2}$ side cover with the Typical Section dimensions adjusted.

ESTIMATED QUANTITIES FOR C-I-P						
ITEM UNIT QUANTITY QUANTITY (36") (42")						
Concrete	CY/LF	0.376	0.420			
Reinforcing Steel (Typical) (excludes Bars 5C & 5F)	LB/LF	62.45	82.17			
Additional Reinf. @ Expansion Joint (Steel Dowels)	LB	21.36	21.36			

(The above concrete quantities are based on a max. superelevation of 6.25%)



OPTIONAL NOTCH AT TOP OF COPING



BUILDUP FOR STEPPED MSE WALL PANELS AND C-I-P COPING

SINGLE-SLOPE TRAFFIC RAILINGS

REVISION 11/01/17

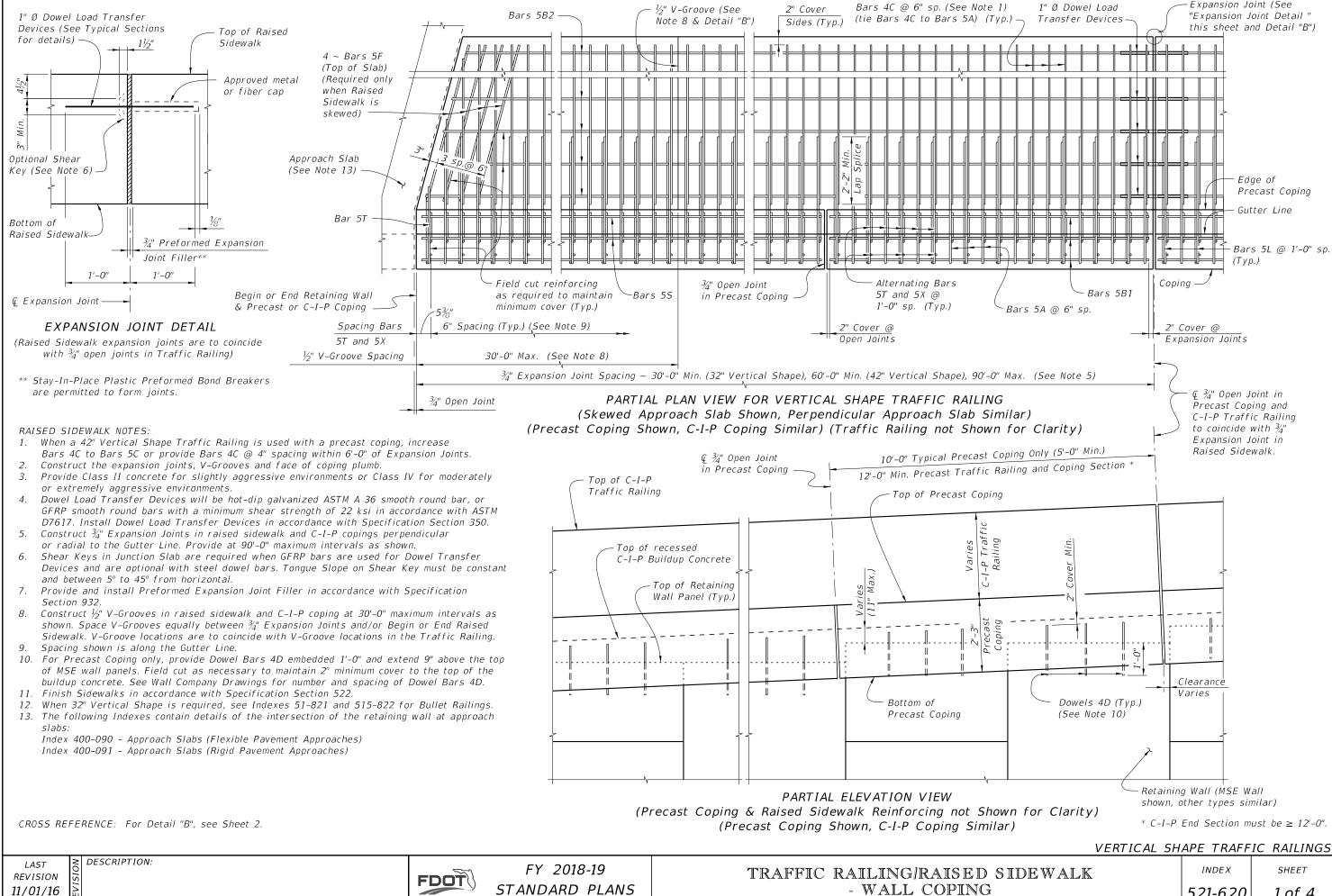


FY 2018-19 STANDARD PLANS

NOTES:

TRAFFIC RAILING/JUNCTION SLAB - WALL COPING

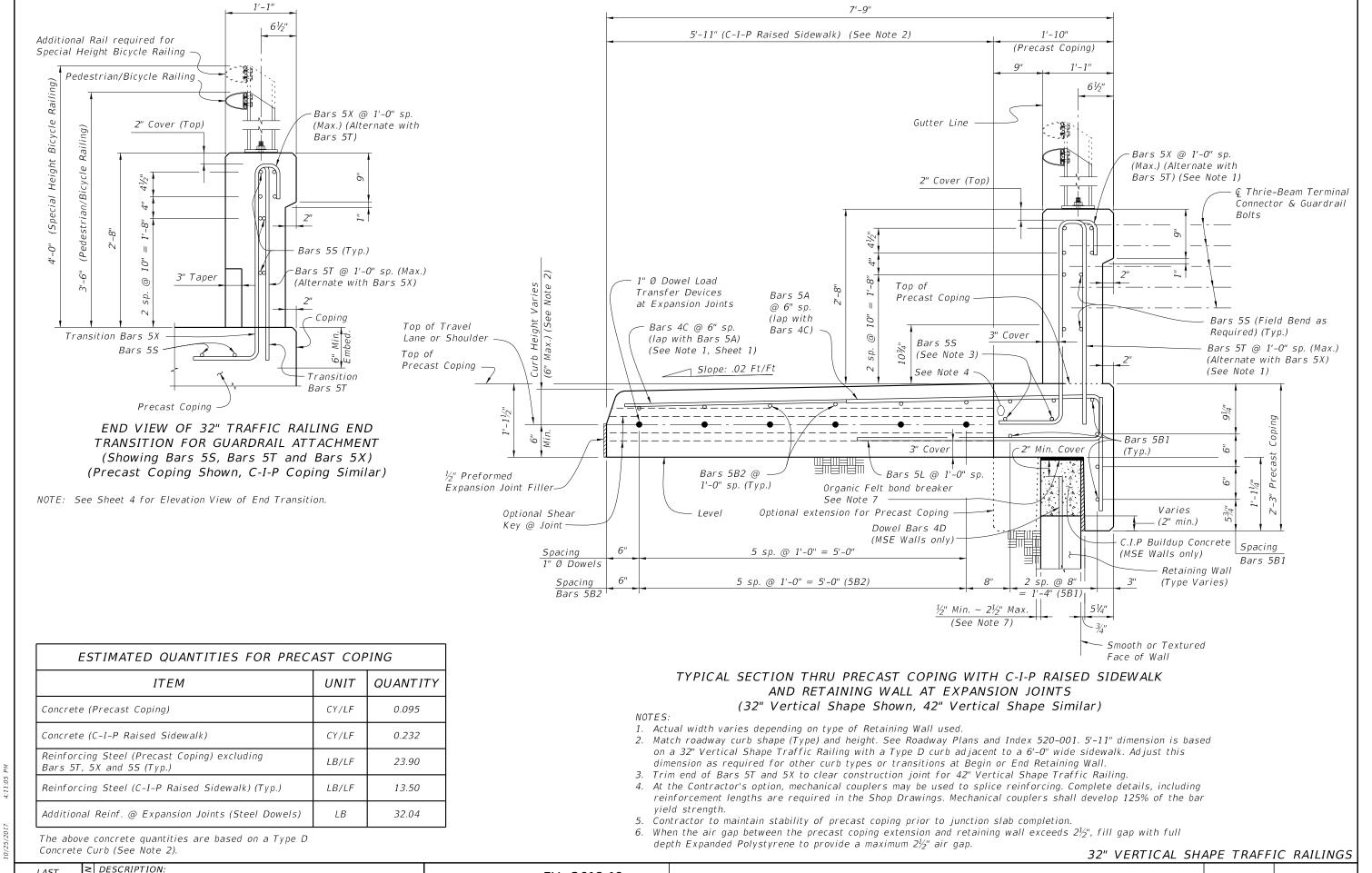
INDEX SHEET *521-610* 3 of 3



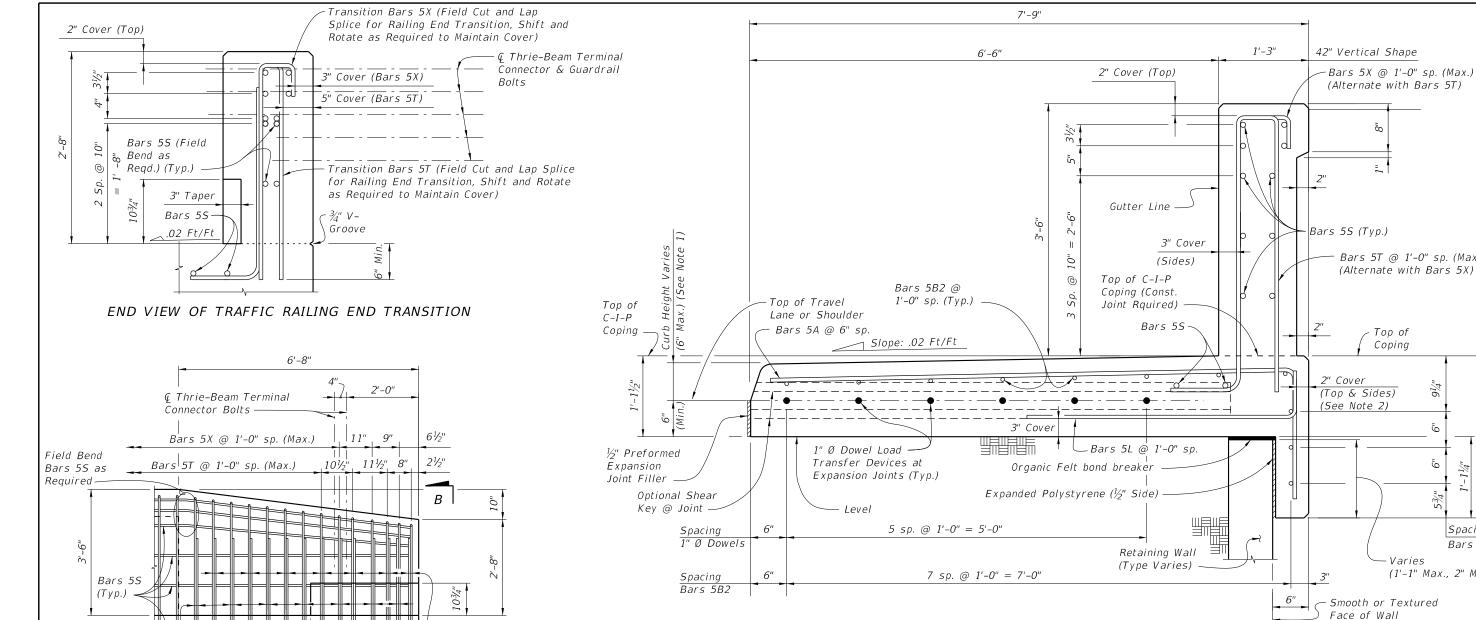
STANDARD PLANS

- WALL COPING

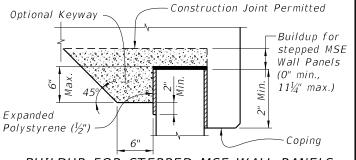
521-620 1 of 4



REVISION 11/01/16



TYPICAL SECTION THRU C-I-P COPING AND RAISED SIDEWALK AND RETAINING WALL AT EXPANSION JOINTS (42" Vertical Face Shown, 32" Vertical Face Similar)



BUILDUP FOR STEPPED MSE WALL PANELS AND C-I-P COPING

- with a Type D curb adjacent to a 6'-0" wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.
- 3. Begin placing Railing Bars 5T and 5X at the railing end and proceed toward Retaining Wall to avoid conflict with guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Cut, shift and rotate Bars 5T and

42" VERTICAL SHAPE TRAFFIC RAILINGS

Concrete Curb on a level Retaining Wall (See Note 1). LAST **REVISION** 11/01/16

FDOT

Transition Bars 5T

(2'-2" Min.)

Approach Slab

Field Cut, Lap Splice

FY 2018-19 STANDARD PLANS

- WALL COPING

INDEX *521-620*

Bars 5T @ 1'-0" sp. (Max.)

(Alternate with Bars 5X)

Varies

Top of

Coping

SHEET 3 of 4

Spacing

(1'-1" Max., 2" Min.)

Bars 5B2

NOTES:

- 1. Match roadway curb shape (Type) and height. See Roadway Plans and Index 520-001. 6'-8" dimension is based on a 32" Vertical Shape Traffic Railing
- 2. If slip forming is used, submit shop drawings for approval showing 3" side cover with the Typical Section dimensions adjusted.
- 5X as required to maintain cover in Railing End Transition.

(Steel Dowels)

ITEM

Reinforcing Steel (Typical) excluding

Additional Reinf. @ Expansion Joints

Bars 5T, 5X and 5S (Typ.)

Transition Bars 5X

Splice (2'-2" Min.) —

Field Cut & Lap

Concrete

Raised Sidewalk

ELEVATION

RAILING END TRANSITION

(Guardrail Not Shown For Clarity)

UNIT

CY/LF

LB/LF

ESTIMATED QUANTITIES FOR C-I-P COPING

The above concrete quantities are based on a Type D

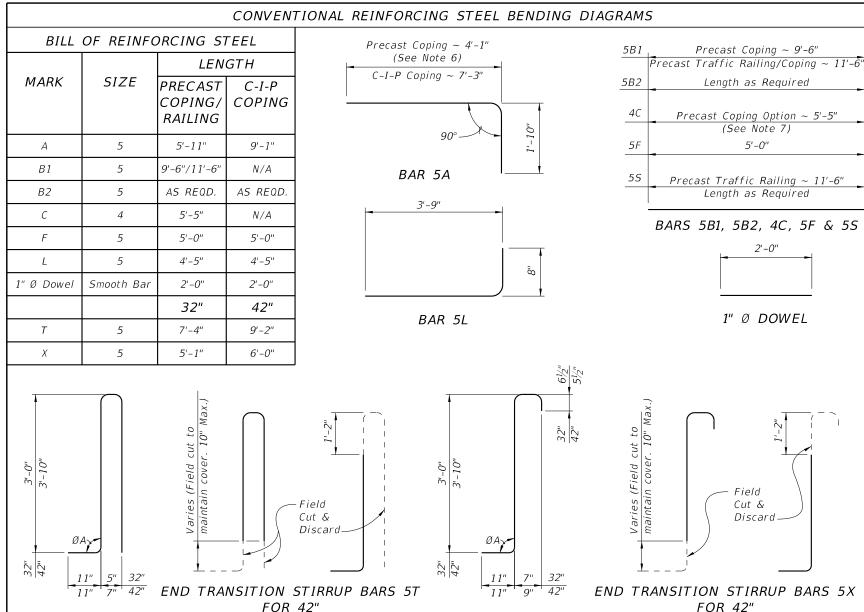
3'-0" Taper

QUANTITY

0.326

35.38

32.04



To Be Field Cut (7 of each required per Railing End Transition)

STIRRUP BAR 5X

END TRANSITION STIRRUP BARS 5X FOR 42"

To Be Field Cut (7 of each required per Railing End Transition)

Field

Cut &

Discard -

5'-0"

2'-0"

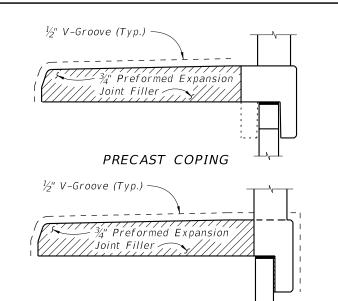
REINFORCING STEEL NOTES:

STIRRUP BAR 5T

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at expansion joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B and 5S will be a minimum of 2'-2".
- 4. Lap splice Bars 5A with Bars 4C. Lap splices will be a minimum of 2'-2".
- 5. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 1'-8".
- 6. Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 5'-8", and reinforcing size must be increased to #5 bars (Bars 5C).
- 7. The Contractor may use deformed WWR when approved by the Engineer. WWR must meet the requirements of Specification Section 931.

* See Sheet 3 Note 3.

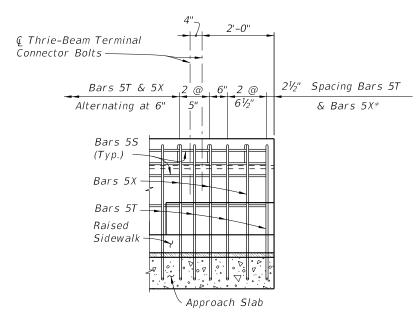
DESCRIPTION:



DETAIL "B"

C-I-P COPING

(Showing Locations of 1/2" V-Grooves and 3/4" Preformed Expansion Joint Filler)



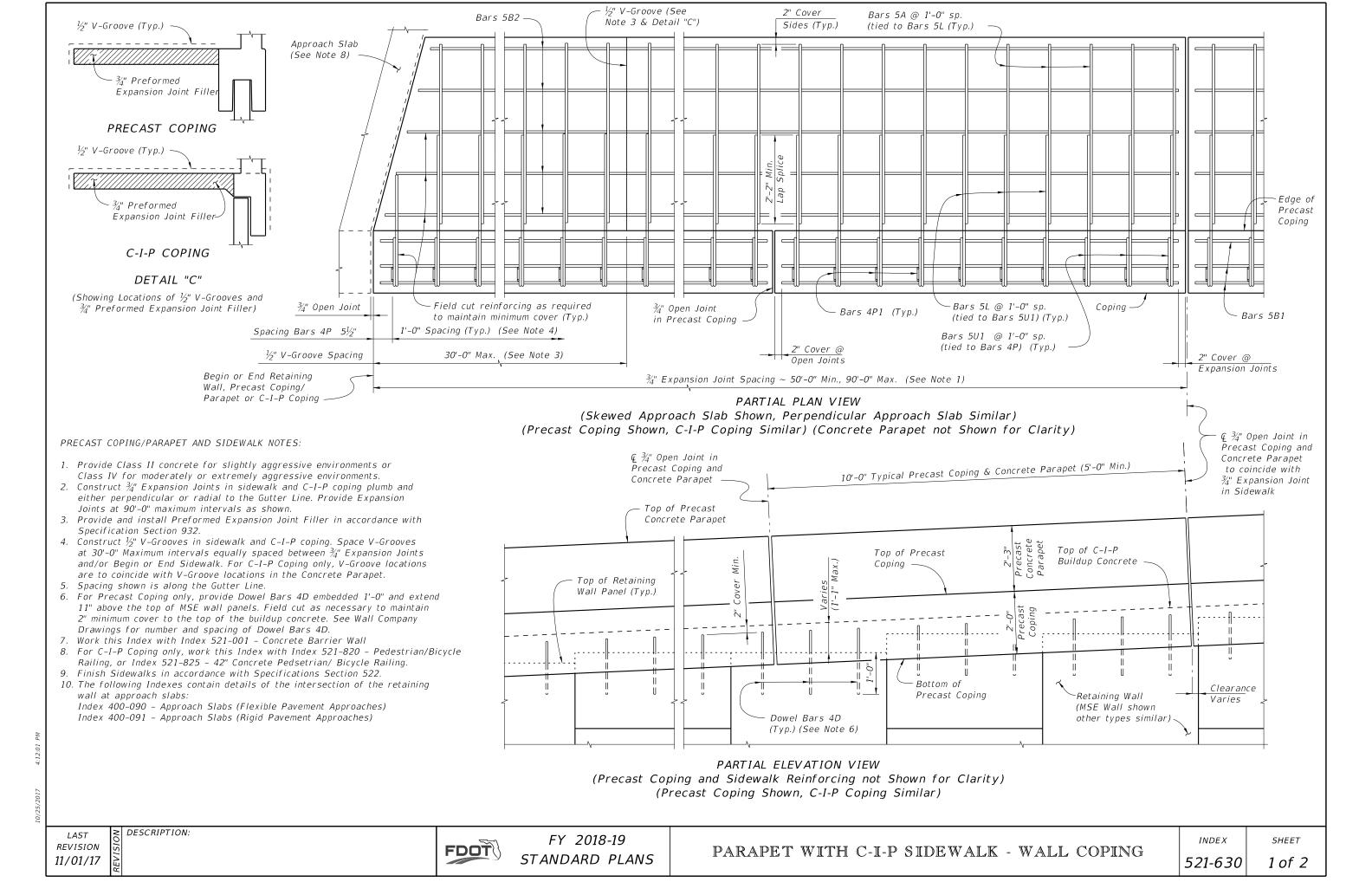
RAILING END DETAIL FOR 32" VERTICAL SHAPE (Guardrail Not Shown For Clarity)

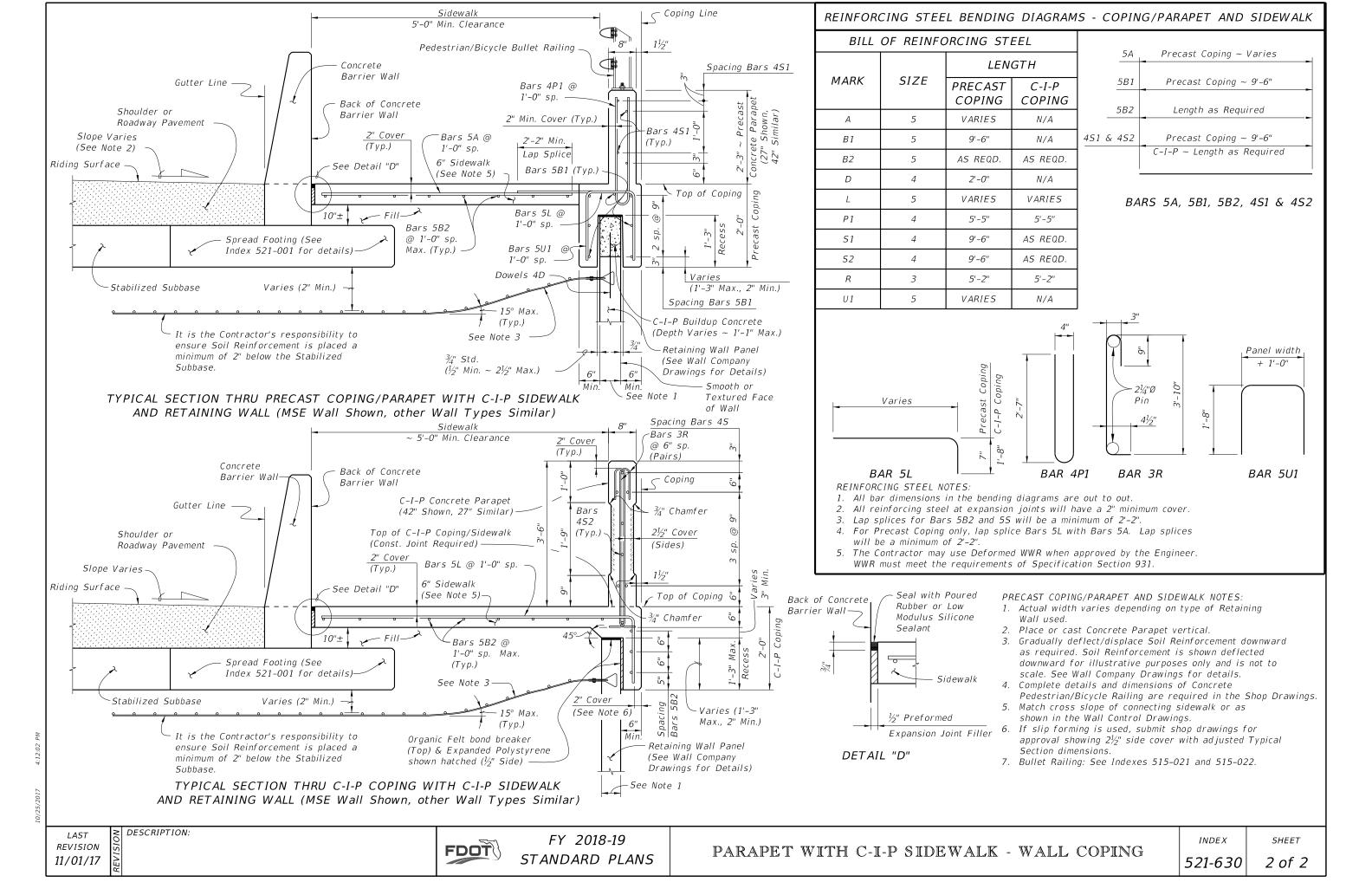
ESTIMATED TRAFFIC RAILING QUANTITIES					
		QUANTITY			
ITEM	UNIT	32"	42"		
Concrete	CY/LF	0.095	0.145		
Reinforcing Steel	LB/LF	23.38	28.33		

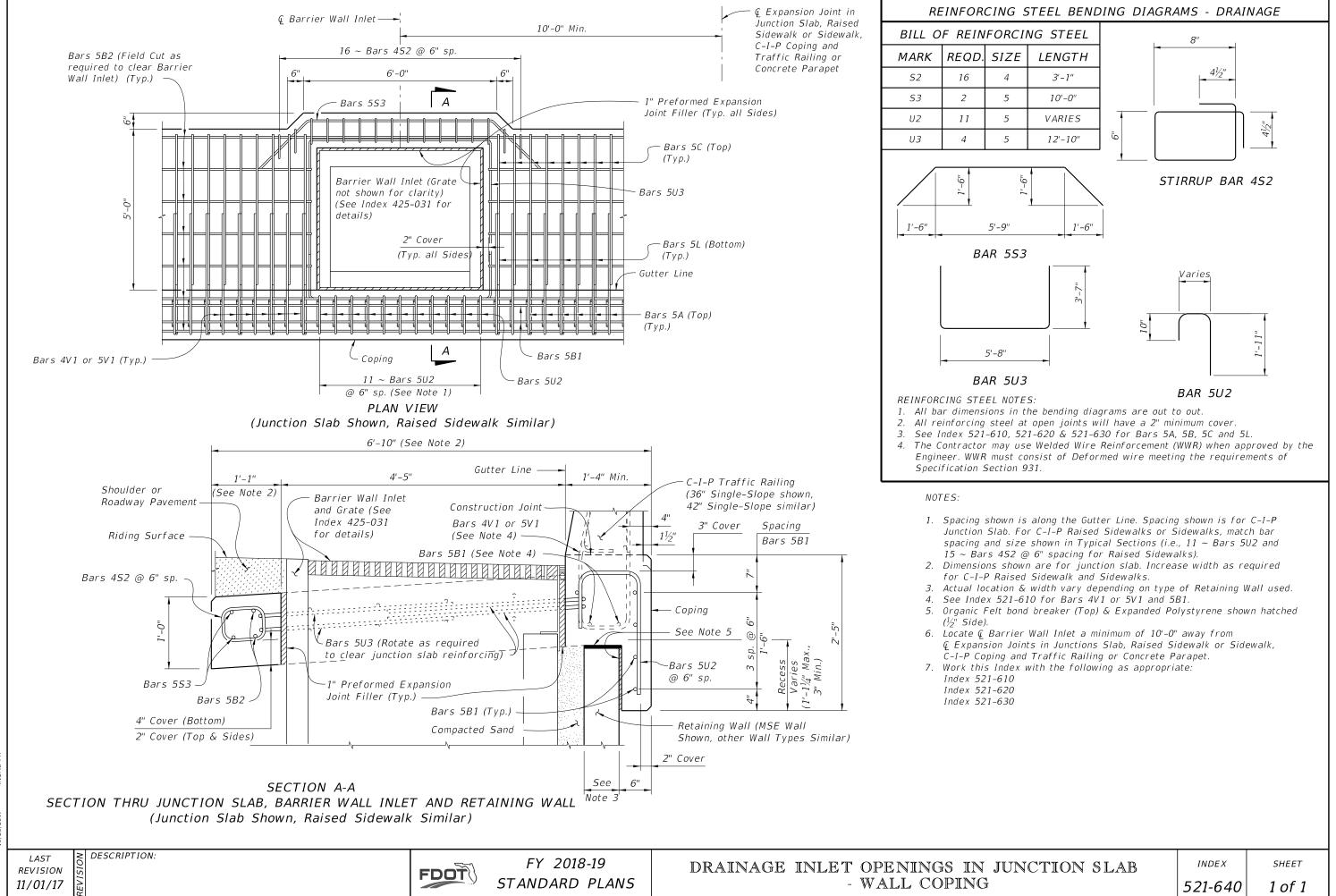
VERTICAL SHAPE TRAFFIC RAILINGS

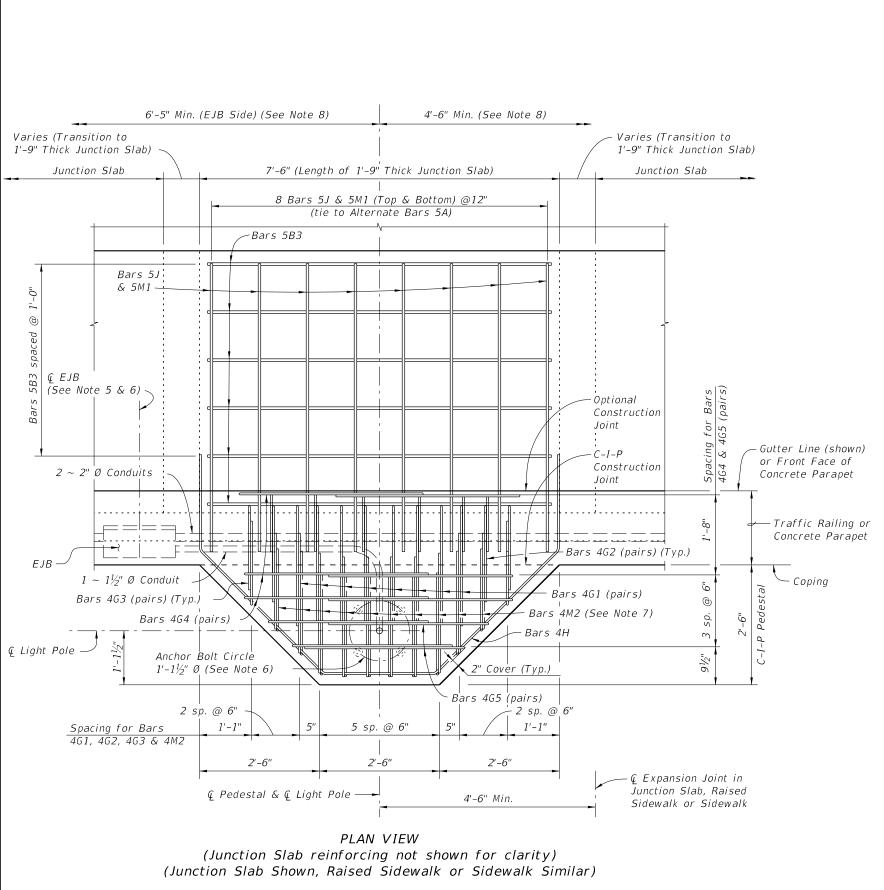
LAST **REVISION** 11/01/17











LIGHT POLE PEDESTAL NOTES:

1. ANCHOR BOLTS:

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole configurations shown on Index 715-040 with top of pedestal 75' or less above ground or MLW.

Anchor Bolt Diameter: See Table 1

2. MATERIALS:

Anchor Bolts: ASTM F1554 Grade 55.

Nuts: ASTM A563 Grade A, Heavy-Hex.

Washers: ASTM F436 Type 1.

Anchor Plate: ASTM A709 (Grade 36) or ASTM A36.

Coating: Galvanize all Nuts, Bolts Washers, and plates in accordance with ASTM F2329.

- 3. The Contractor is responsible for ensuring the anchor bolt design is compatible with the light pole base plate. Modifications to the anchor bolt design shown must be signed and sealed by the Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.
- 4. Install Anchor Bolts plumb.
- For conduit, EJB and expansion/deflection fitting details, see Utility Conduit Detail Drawings.
- 6. The cost of anchor bolts, nuts, washers and anchor plates will be included in the Bid Price for Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Traffic Railing or Concrete Parapet that the pedestal is behind.
- 7. Field Cut Bars 4M2 as required to maintain clearance.
- 8. Slip Forming Method of construction requires the Engineer's approval within the limits shown.
- 9. Reinforcing shown for light pole pedestals is in addition to typical reinforcing for Junction Slabs and Raised Sidewalks.
- 10. Work this Index with the following as appropriate:

Index 521-512 Index 521-610

Index 521-610

Index 521-630

- 11. Pedestal may be precast in one section with Coping. Minimum Precast Coping section length is 10 ft. or 12 ft for combination Precast Traffic Railing and Coping section.
- 12. For Estimated Quantities, see Sheet 6.
- 13. Unless otherwise noted, Traffic Railing (36" Single-Slope) is shown in all Views and Sections. The Pedestal details for other traffic railings or pedestrian/bicycle railings are similar.

TABLE 1 DESIGN LIMITATION FOR ANCHOR BOLTS (1" Dia.)					
Wind Speed					
(MPH)	(FT)	40 ft.	45 ft.	50 ft.	
120	ALL	75	75	75	
140	ALL	75	75	75	
160	8 & 10	75	75	45**	
160	12 & 15	75	75	25*	

- * Above Natural Ground
- ** Use $1\frac{1}{4}$ " Ø Anchor bolts for wall heights greater than the height shown and less than 75'.

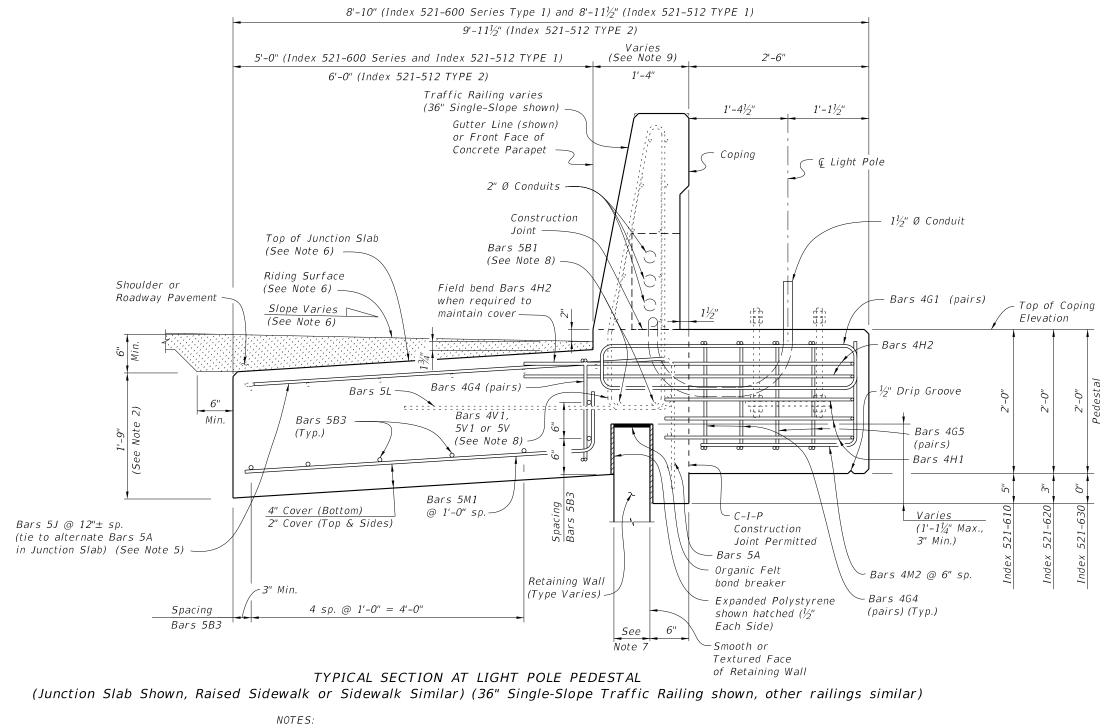
LAST REVISION 11/01/17

FDOT

FY 2018-19 STANDARD PLANS

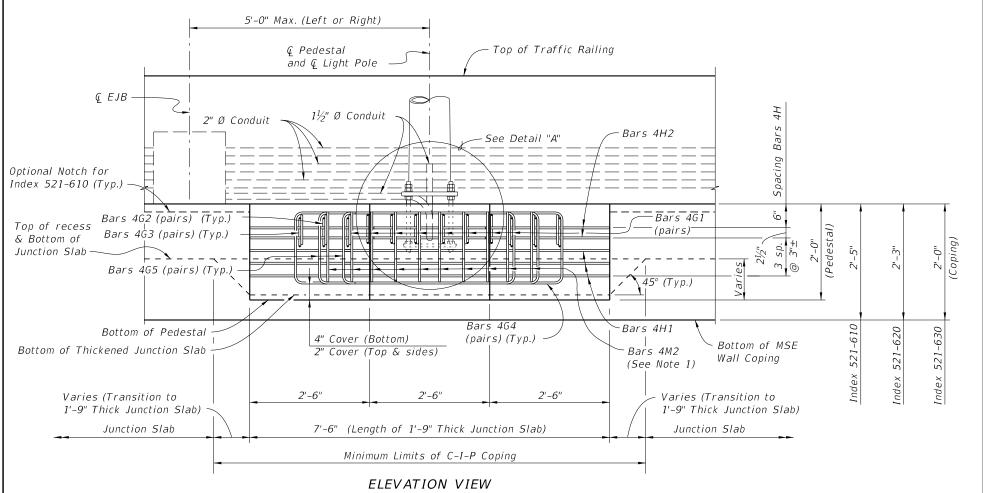
LIGHT POLE PEDESTAL - WALL COPING

INDEX **521-650**

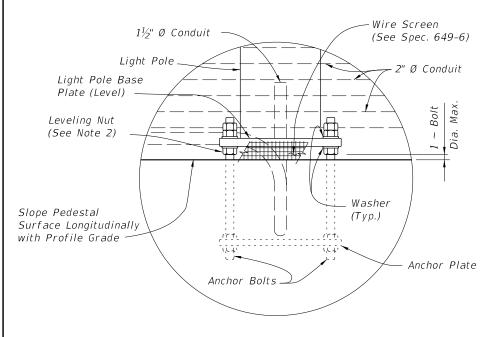


- 1. Provide Concrete Class to match adjacent coping.
- For junction slabs, increase the 1'-0" depth dimension to 1'-9".
- For Parapet with sidewalk see Index 521-630, but increase 6" sidewalk depth to 1'-6". For raised sidewalk see Index 521-620.
- The minimum length of the Junction Slabs, raised sidewalks and sidewalks is 30'-0", measured along the Gutter Line.
- Bars 4J are only required when pedestals are behind a Traffic Railing or Traffic Railing/ Noise Wall.
- Top of junction slab may be thickened to match finished grade of concrete pavement or shoulder, or top of sidewalk or raised sidewalk (See Notes 3 & 4).
- Actual width varies depending on type of retaining wall used.
- See Index 521-610 for Bars 4V1, 5V1 and 5B, or Index 521-512 for Bars 5V and 5B1.
- Work with Index 521-512 (Traffic Railing/ Noise Wall), Index 521-610 (Single-Slope), Index 521-620 (Vertical Shape), and Index 521-630 (Concrete Parapet).

DESCRIPTION:



(Junction Slab Reinforcing & Bars 4J not Shown for Clarity)
(Junction Slab Shown, Raised Sidewalk or Sidewalk Similar)



NOTES

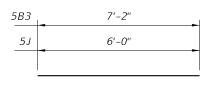
- 1. Field Cut Bars 4M2 as required to maintain minimum cover.
- 2. Maximum clearance between leveling nut and top of pedestal will not exceed anchor bolt diameter.

ESTIMATED QUANTITIES					
ITEM	UNIT	QUANTITY			
Concrete (Pedestal)	CY	0.926			
Concrete (Thickened Junction Slab)	CY	1.222			
Reinforcing Steel	LB	334.09			

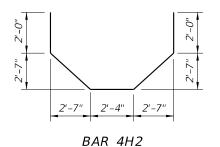
(The quantities above are for one C-I-P Light Pole Pedestal. The concrete quantity for the thickened junction slab is based on a 5'-0" length, 9" increase in thickness and a 5" wide retaining wall panel. Adjust thickened concrete quantity as required.

REINFORCING STEEL BENDING DIAGRAMS - LIGHT POLE PEDESTAL

BILL OF REINFORCING STEEL					
MARK	SIZE	NO. REQD.	LENGTH		
В3	5	7	7'-2"		
G 1	4	16	5'-8"		
G2	4	4	4'-8"		
G3	4	4	4'-2"		
G4	4	6	8'-10"		
G5	4	4	7'-4"		
H1	4	3	9'-8"		
H2	4	2	13'-8"		
J	5	8	6'-0"		
M 1	5	8	5'-10"		
M2	4	10	3'-8"		

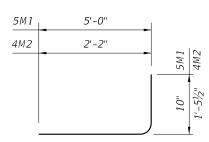


BARS 5B3 & 5J



2'-7" 2'-4" 2'-7"

BAR 4H1



BARS 4G1, 4G2, 4G3, 4G4 & 4G5

BAR 5M1 & 4M2

REINFORCING STEEL NOTES:

2'-6" 2'-0"

1'-9"

3'-8"

2'-11"

1. All bar dimensions in the bending diagrams are out to out.

4G1

4G3

4G4

4G5

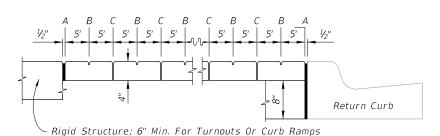
- 2. Lap splices for Bars 4G1, 4G2, 4G3, 4G4 & 4G5 will be a minimum of 1'-4".
- 3. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

DETAIL "A"

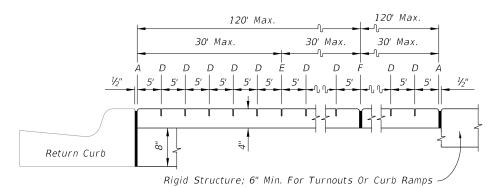


GENERAL NOTES:

- 1. Construct sidewalks in accordance with Specification 522. Use 6" concrete for Sidewalks and Curb Ramps Located within Curb Returns (See Plan View). Install all other concrete with thickness as shown, unless otherwise detailed in the Plans.
- 2. Include detectable warnings on sidewalk curb ramps in accordance with Index 522-002.
- 3. For TURNOUTS see Index 000-515.
- 4. Bond breaker material can be any impermeable coated or sheet membrane or preformed material having a thickness of not less than 6 mils not more than 1/2".
- 5. Construct sidewalks with Edge Beam through the limits of any surface mounted Pedestrian/Bicycle Railing or Pipe Guiderail shown in the plans. (See RAILING DETAIL)
- 6. When roadways or driveways are newly constructed, reconstructed or altered, construct the cross slopes for crosswalks and discontinuous sidewalks as follows:
- A. Max. 0.02 cross slope for roadways or driveway controlled by "STOP" Sign or "YIELD" sign.
- B. Max. 0.05 cross slope for roadways or driveways controlled by traffic signal.



OPEN JOINTS



SAWED JOINTS

LONGITUDINAL SECTION

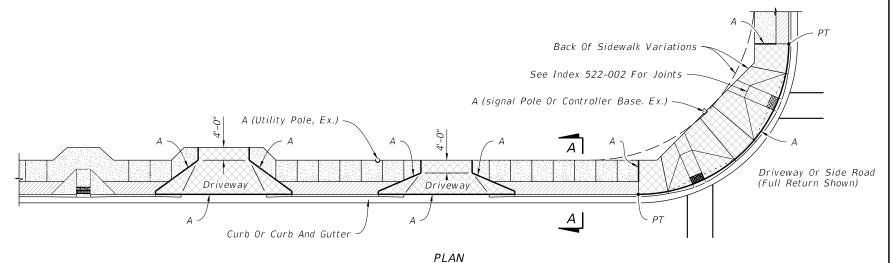
LEGEND:

- A- 1/2" Expansion Joints (Preformed Joint Filler)
- B- $\frac{1}{8}$ " Dummy Joints, Tooled
- C- 1/8" Formed Open Joints

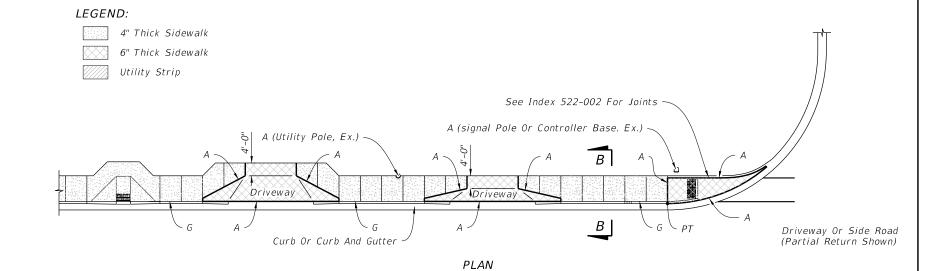
DESCRIPTION:

- $D-\sqrt[3]{_{16}}$ " Saw Cut Joints, $1\frac{1}{2}$ " Deep (within 96 hours) Max. 5' Centers
- E- $\frac{3}{16}$ " Saw Cut Joints, $1\frac{1}{2}$ " Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'
- F- 1/2" Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.
- G- Cold Joint With Bond Breaker, Tooled

= SIDEWALK JOINTS ====



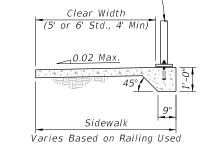
=SIDEWALK WITH UTILITY STRIP=



SIDEWALK WITHOUT UTILITY STRIP=



= SECTION B-B===== SECTION A-A



Railing (See Index 515-052,

515-062, 515-070 or 515-080)

=== RAILING DETAIL ====

GENERAL NOTES AND CONCRETE SIDEWALK ON CURBED ROADWAYS

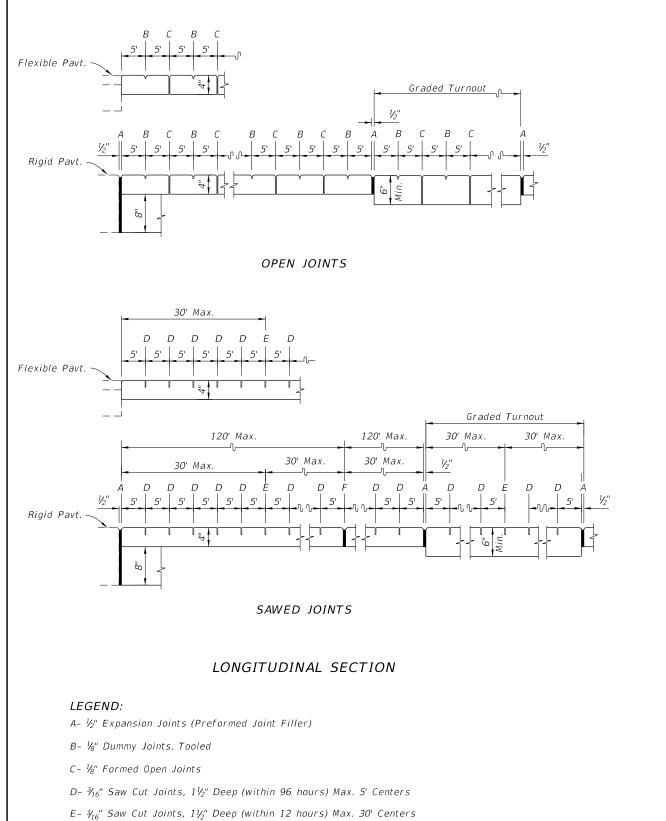
REVISION 11/01/17

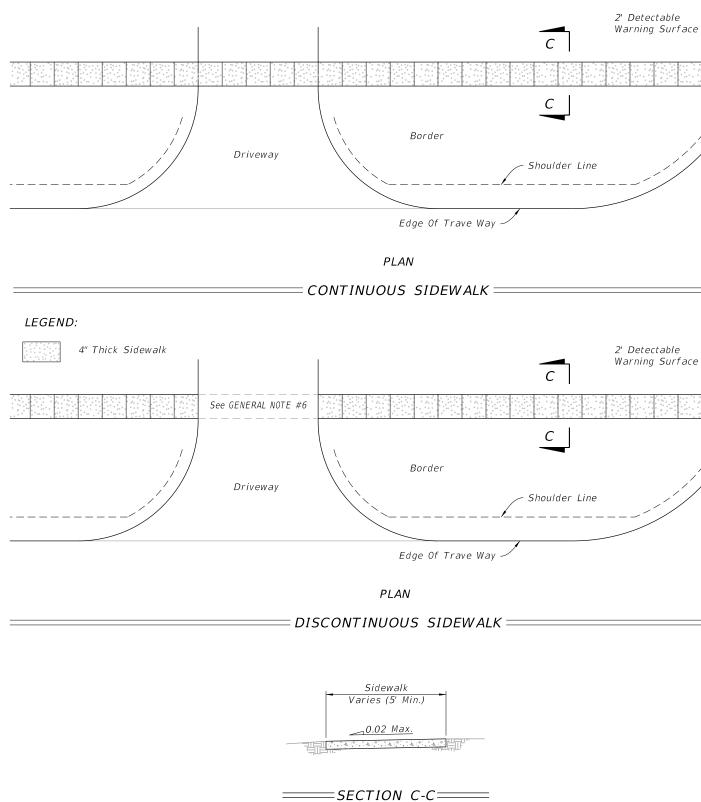
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FY 2018-19 STANDARD PLANS

INDEX *522-001*

1 of 2







10/23/2017

LAST ODESCRIPTION:

11/01/17

Joint(s) Required When Length Exceeds 30'

F- $\frac{1}{2}$ " Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by

= SIDEWALK JOINTS ==

FDOT

FY 2018-19 STANDARD PLANS CONCRETE SIDEWALK ON FLUSH SHOULDER ROADWAYS

Side Road

Side Road

- A. Sidewalk, ramp, and landing slopes (i.e. 0.02, 0.05, and 1:12) shown in this Index are maximums. With approval of the Engineer, provide the minimum feasible slope where the requirements cannot be met.
- B. Landings must have cross-slopes less than or equal to 0.02 in any direction.
- C. Install ramp slopes along a single linear plane (i.e. no warps or varying slope). Ramp slopes are not required to exceed 15 feet in length.
- D. Joints permitted at the location of Slope Breaks. Otherwise locate joints in accordance with Index 522-001. No joints are permitted within the ramp portion of the Curb Ramp.

2. <u>Grade B</u>reaks:

Grade breaks at the top and bottom of ramps must be parallel to each other and perpendicular to the direction of the ramp slope.

3. Curb, Curb and Gutter and/or Sidewalk:

- A. Refer to Index 522-001 for concrete thickness and sidewalk details.
- B. Remove any existing curb or curb and gutter to the nearest joint beyond the curb transition or to the extent that no remaining section of curb or curb and gutter is less than 5 feet long. Remove any existing sidewalk to the nearest joint beyond the transition slope or to the extent that no remaining section of sidewalk is less than 5 feet long.

4. Curb Ramp Alpha-Identification:

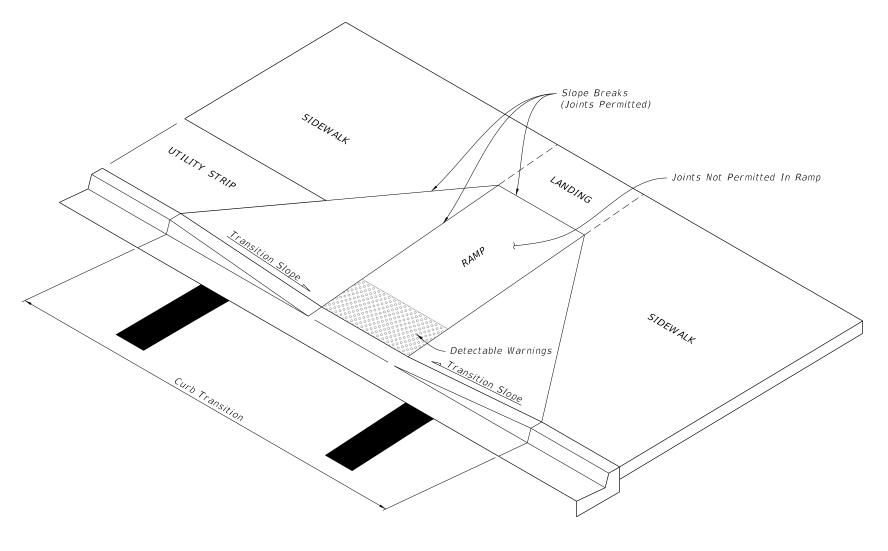
- A. Sidewalk curb ramp alpha-identifications (e.g. CR-A) are provided for reference purposes in the Plans.
- $B. \ Alpha-identifications \ CR-I \ and \ CR-J \ are \ intentionally \ omitted.$

5. Detectable Warnings:

- A. Install detectable warnings in accordance with Specification 527.
- B. Place detectable warnings across the full width of the ramp or landing, to a minimum depth of 2 feet measured perpendicular to the curb line and no greater than 5 feet from the back of the curb or edge of pavement.
- C. If detectable warnings are shown in the Plans on slopes greater than 5%, align the truncated domes with the centerline of the ramp; otherwise, the truncated domes are not required to be aligned.

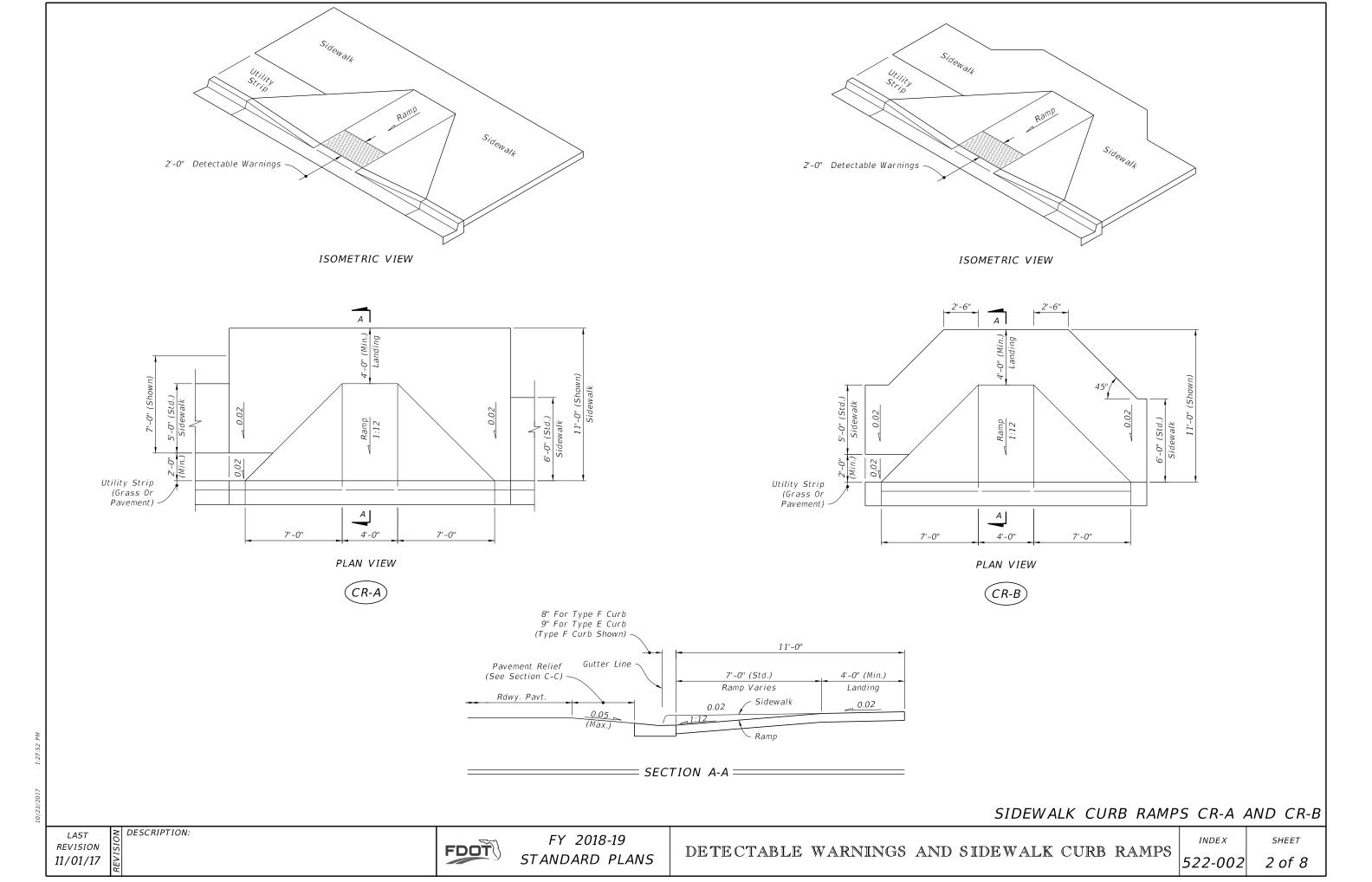
6. Detectable Warnings - Acceptance Criteria:

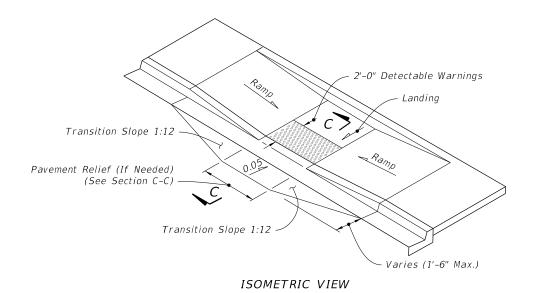
- A. Color and texture shall be complete and uniform.
- B. 90% of individual truncated domes shall be in accordance with the Americans with Disabilities Act Standards for Transportation Facilities, Section 705.
- C. There shall be no more than 4 non-compliant domes in any one square foot.
- D. Non-compliant domes shall not be adjacent to other non-compliant domes.
- E. Surfaces shall not deviate more than 0.10" from a true plane.

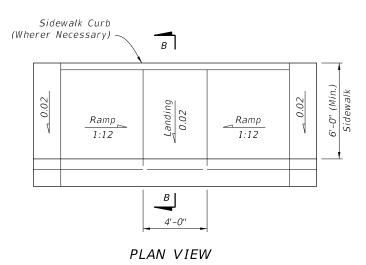


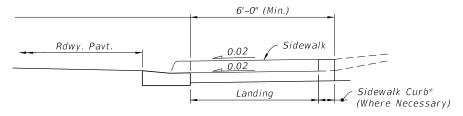
CURB RAMP NOMENCLATURE =

10/23/2017



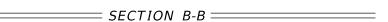


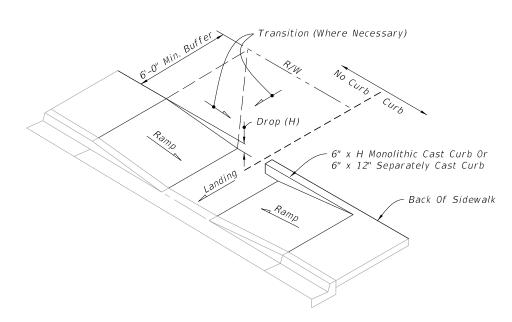




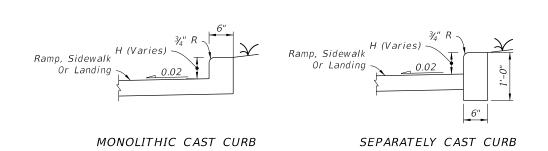
(CR-C)

* Note: For Additional Information On Sidewalk Curb Construction, See SIDEWALK CURB OPTIONS details.

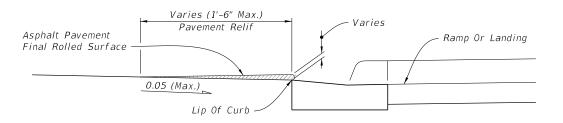




CONSTRUCTION OF SIDEWALK CURB IN CUT SECTIONS



SIDEWALK CURB OPTIONS:



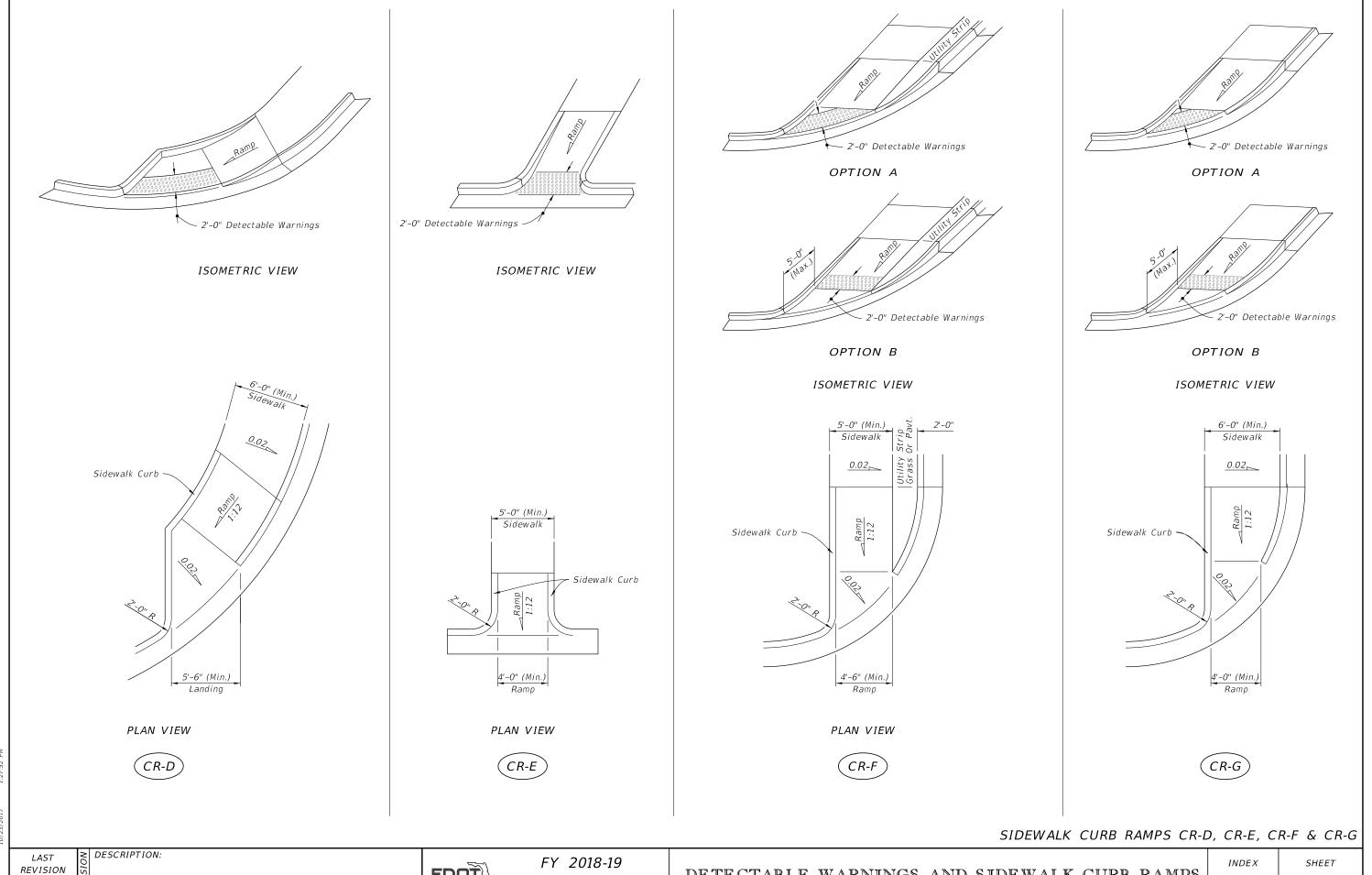
Note: Remove Elevated Pavement By Spading And Rolling, Smooth Milling or Grinding.

= SECTION C-C ====

SIDEWALK CURB RAMPS CR-C AND SIDEWALK CURB

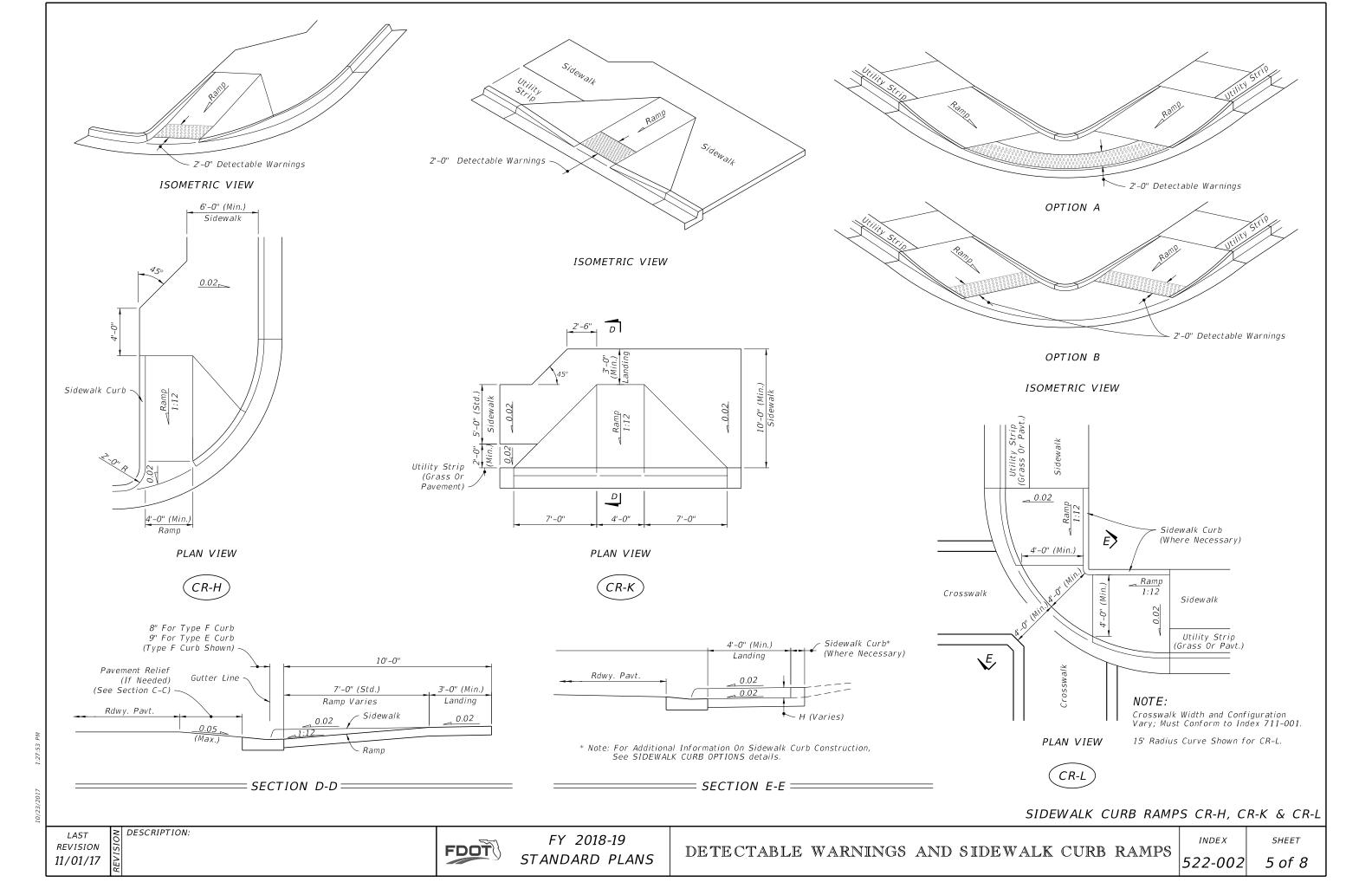
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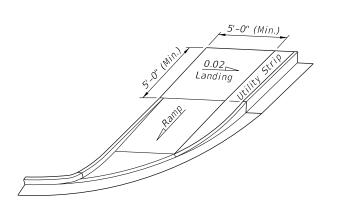
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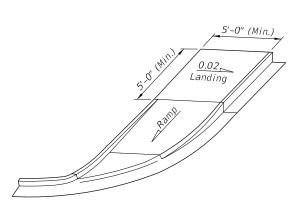


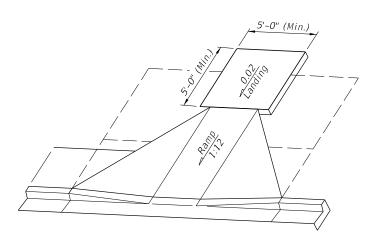
11/01/17

FDOT



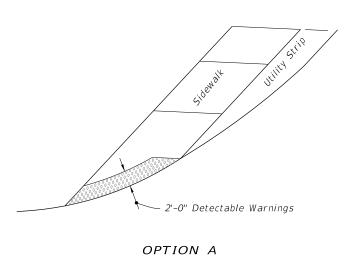


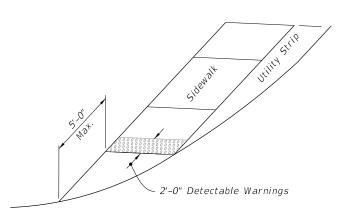




LANDINGS FOR CURB RAMPS WITHOUT SIDEWALKS

(See CR-F, CR-G & CR-K Respectively For Detectable Warning Details/Options)





OPTION B

DETECTABLE WARNING ON FLUSH SHOULDER SIDEWALKS =

CURB RAMPS WITHOUT SIDEWALKS AND FLUSH SHOULDER SIDEWALKS

LAST DESCRIPTION:
REVISION II/01/17

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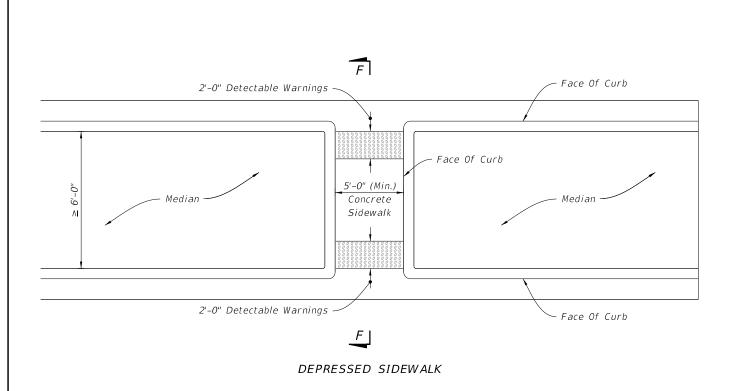
DETECTABLE WARNINGS AND SIDEWALK CURB RAMPS

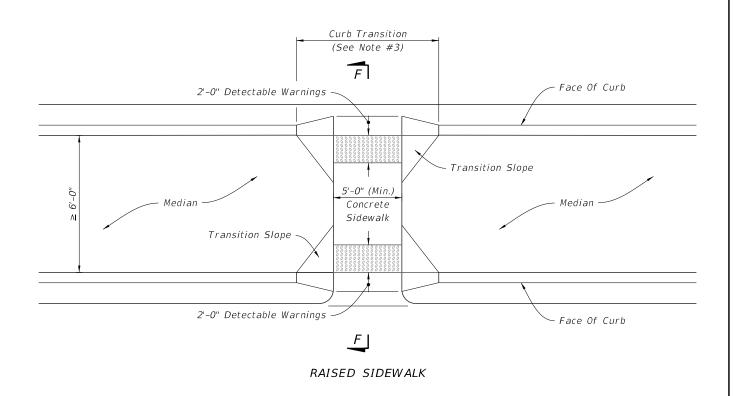
522-002

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SHEET

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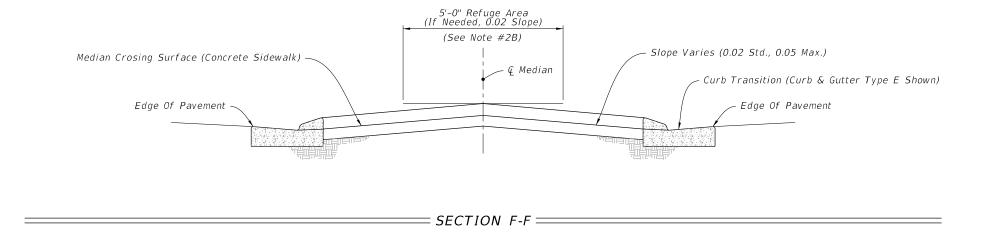




= MEDIAN CROSSINGS =

NOTES:

- 1. Cross Slope of the median crossing not to exceed 0.02.
- 2. Running Slopes:
 - A. Slopes ≤ 0.05: For roadway cross sections were the Edge of Pavement elevation is the same for both directions of traffic, the median crossing running slopes (0.02 Typ.) should meet at the centerline of the median. For roadway cross sections with variable Edge of Pavement elevations, or to accommodate other construction in the median, the slopes may intersect off the centerline of the median.
 - B. Slopes > 0.05: Provide a median refuge area (landing, 0.02 slope) for crossings with running slopes > 0.05. The refuge area must extend the full width of the crossing and have a minimum length of 5 feet.
- 3. On existing facilities, remove and reconstruct curb transition for raised sidewalk with ramp.



MEDIAN CROSSING

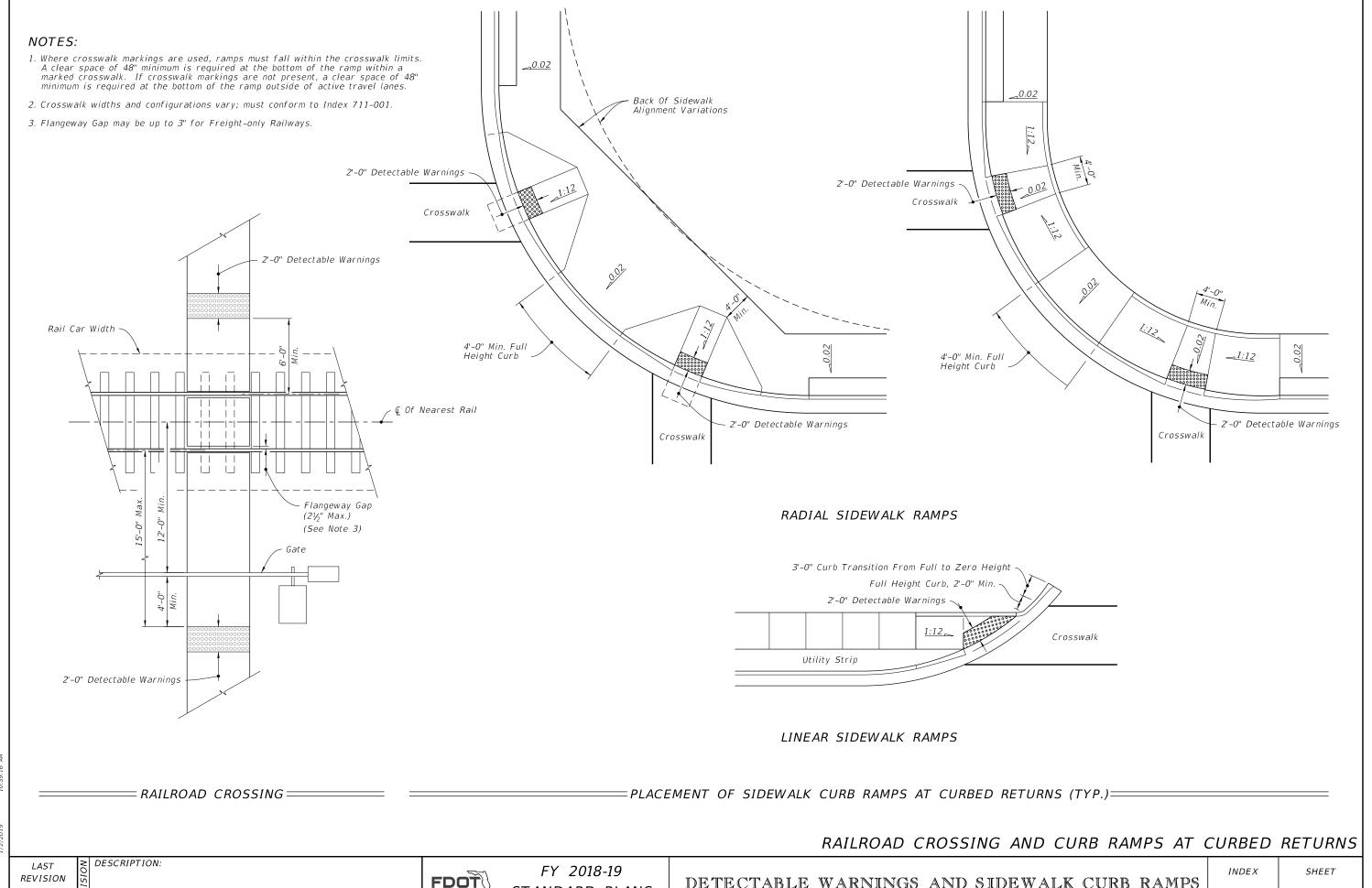
LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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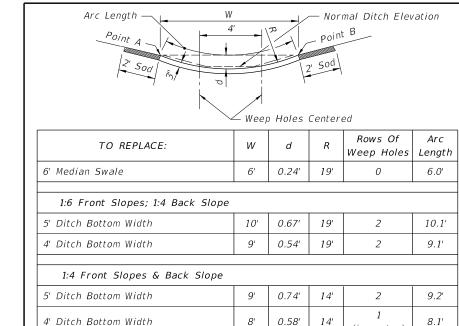
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STANDARD PLANS

DETECTABLE WARNINGS AND SIDEWALK CURB RAMPS

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For use only where side slopes are 1:4 or flatter. Point "A" and "B" are to be the same elevation and should be used to locate the paved section.

≣5' Min.**≡**

Do Not Construct Weep Holes In

This Area Or 5' Upstream

- 1·1 5 Slone

- Roadway Ditch

- Back Slope As

Shown On Plans

Ditch Slope

construction.

* Misc. asphalt will not be

permitted for this type of

ALTERNATE DITCH PAVEMENT

Front Slope

JUNCTION OF ROADWAY DITCH*

AND LATERAL DITCH

5' Varies 5'

DESCRIPTION:

Min.

SECTION MATTING FOR DITCH

50' Max. Erosion Stops

6" Typical

- Side Slope

PLAN

Matting -

LONGITUDINAL SECTION

— 6" Overlap

Note: All weep holes to be 3"x4" rectangle or 4" or 5" dia. circle hole. V_2 cu. ft. (12" x 12" x 6") of No. 6 aggregate to be placed under each hole. 1 sq. ft. of galv. wire mesh ($\frac{1}{4}$ " openings) shall be placed between the aggregate and the ditch pavement. Cost of holes, aggregate and wire mesh to be included in the cost of ditch navement.

When Width Is Greater Than 4',

One Row

When "x"= 1' To 4' Const. 1 Row (Centered)

"x"= 5' To 7' Const 2 Rows

"x"= 8' To 12' Const. 3 Rows

"x"= 13' To 17' Const. 4 Rows

"x"= 18' To 22' Const. 5 Rows

ROADWAY SIDE DITCH

40'

Std.

Sod Or Ditch Pavt.

40' MEDIAN

* Filter Fabric Required.

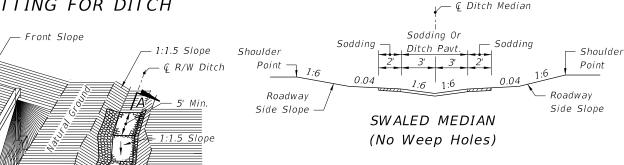
Back Slope Front

Slope

Const. Weep Holes Half-Way Up The

Side In Line With Bottom Weep Holes

WEEP HOLE ARRANGEMENT



10' C. to C.

Staples Not More Than 3' Centers

6" Min. Overlap

One Row Of Staples

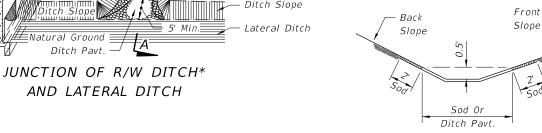
Each Edge Of Overlaps,

Each Side Of Stops And

On Outer Edges At Not

More Than 18" Centers

(Typical)



Ditch Width Varies Normal Ditch Elev. Front And Back Front And Back Slopes Vary 3"x4" Weep Holes —

TYPICAL SECTION

SECTION AA PROFILE OF DITCH PAVEMENT

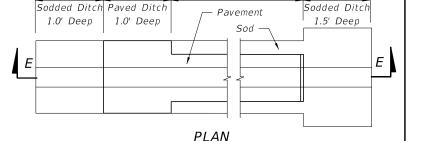
Ditch Grade

3"x4" Weep Holes

AT LOCATIONS OTHER THAN HINCTION WITH LATERAL DITCH

(in center)

AI LOCATIO	145		<u>, , , , </u>		IMIV JUI	VCTIOIV	VVIIII LAI	LIVAL DITCH				
TABLE 1: DITCH PAVEMENT												
Pavement Type Dimensions Payment Basis Of Filter Fabric Velocity References & Remarks												
ravement Type	а	b	С	Unit	Estimate	Туре	Range	References & Remarks				
Concrete	24"	6"	3"	5Y	SY	D-4*	Low-High	Section 524 of the Standard Specifications.				
Miscellaneous Asphalt	24"	12"	4"	TN	0.2 TN/SY	None	Low-Moderate	Section 339.				
Riprap (Sand-Cement)	24"	12"	4"	CY	0.11 CY/SY	D-4*	Low-Moderate	Section 530. Grouting of joints required.				
Riprap (Ditch Lining)				TN	TN	D-2*	Moderate-High	Section 530.				



SECTION EE

Lip (3" Rise)

Standard Paved Ditch

Flow Line

10'

10'

— Varies (25' Min.)

10'

PAVED DITCH END TREATMENT

GENERAL NOTES

- 1. Type of ditch pavement shall be as shown on plans.
- 2. In concrete ditch pavement, contraction joints are to be spaced at 25' maximum intervals, or as directed by the Engineer. Contraction joints may be either formed (construction joint) or tooled. No open joints will be permitted in concrete ditch pavement.

Expansion joints with $\frac{1}{2}$ " preformed joint filler shall be constructed at all inlets, endwalls, and at intervals of not more than 200'.

- 3. Lip at end of ditch pavement shall normally be located downstream of DPI or on flatter grades where there is a decrease in ditch velocity.
- 4. Toewalls are to be used with all ditch paving. A toewall is not required adjacent to drainage structures.
- 5. When directed by the Engineer, weep hole spacing may be reduced to 5' minimum.
- 6. For junction of R/W ditch spillway and lateral ditch, sides of paving to be 1' high minimum.
- 7. For ditch pavements requiring filter fabric (See Table 1) place the filter fabric directly beneath the pavement for the entire length and width of the pavement. See Standard Specification Section 985 for fabric requirements and application.
- When weep holes with aggregate are used, place filter fabric below the aggregate to form a mat continuous with the pavement filter fabric or underlapping the pavement filter fabric, if present.
- 9. Ditch pavement requiring reinforcement shall be detailed in the plans.
- 10. Cost of plastic filter fabric to be included in the contract unit price for ditch pavement.
- 11. Sodding to be paid for under contract unit price for Performance Turf, SY

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Ditch

Grade -



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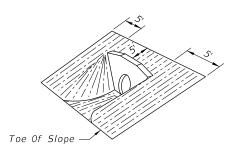


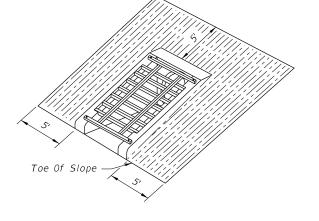
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Note: Sodding quantities for each endwall to be determined by the designer from this detail.

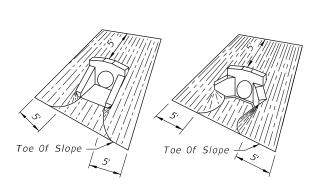




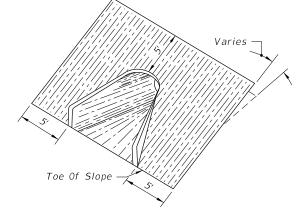
(EXCEPT INDEX 430-030) STRAIGHT ENDWALL

STRAIGHT ENDWALL INDEX 430-030

U-TYPE ENDWALL INDEX 430-011



U-TYPE WINGS 45° WINGS WINGED ENDWALLS INDEX 430-040



FLARED END SECTION INDEX 430-020

	TABLE 2: SOD QUANTITIES (SY)																				
					IN	DEX .	430-0	30					INL	IDEX 430-011 INDEX 43						10	INDEX 430-020
PIPE	E SLOPE										SLOF	PE			SLO	OPE	ALL SLOPES				
SIZE	?E 1:2 1:3 1:4 1:6						1:2	1:3	1:4	1:6	1:2	1:3	1:4	1:6	ALL SLOTES						
	PIPES							·	PIPE	S			PIF	PES		PIPES					
	1	2	3	1	2	3	1	2	3	1	2	3	1	1	1	1	1	1	1	1	1
12"																	14	15	18	22	10
15"	19	21	24	22	26	29	26	30	33	34	38	43	13 (15)	16	17	23	15	17	20	25	11
18"	21	24	27	25	29	33	30	34	38	39	44	50	14 (16)	17	19	25	16	18	22	28	11
21"																					12
24"	26	30	34	32	37	42	38	44	50	50	58	66	15 (17)	19	21	28	19	22	26	34	14
27"																					15
30"	31	37	42	39	46	53	46	55	63	62	74	85	17 (18)	21	24	32	21	25	30	40	16
36"	37	44	52	46	56	65	56	67	79	76	91	107					24	29	35	47	18
42"	43	53	62	55	67	79	67	82	96	91	111	132					27	32	39	54	19
48"	50	62	73	64	79	93	78	97	115	108	133	158					30	36	44	61	21
54"	57	71	85	74	92	110	91	113	136	126	157	188									21
60"																					22
66"																					25
72"																					26
	() E											() Endw	all Wi	th Bat	fles						

SOD PLACEMENT AT PIPE/CULVERT END TREATMENTS

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DESCRIPTION:

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INDEX DITCH PAVEMENT AND SODDING *524-001* 2 of 2

Sand-Cement Or

Sand-Cement Or Rubble Riprap

Rubble Riprap

12" Bituminous Coating

On Face Of Concrete

1"x2" Pressure Treated Timber Nailed To Surface

BONDED OPTION

NAILED OPTION

FILTER FABRIC PLACEMENT AT CONCRETE STRUCTURE

Note: Either option may be used unless otherwise called for in the plans.

6" Fold Min.

No Adhesive Above Here

NOTES

- 1. Work this Index with the Noise Wall Data Tables, and Wall Control Drawings in the Plans.
 - A. Prestressed concrete posts with equivalent strength resistance may be substituted for conventionally reinforced precast posts shown in this index when approved as part of a Producer's Quality Control Plan.
 - B. Producer shop drawings for prestressed concrete post designs must be approved by the State Structures Design Office prior to inclusion in the Quality Control Plan.
- 2. Construct Noise Walls in accordance with the requirements of Specification Section 534, and Augers Cast Piles in accordance with Specification Section 455.
- 3. Field verify the location of all overhead and underground services shown in the Wall Control Drawings.
- 4. Wall Height is the nominal height of the walls above finished grade. The Wall Embedment Depth for design is 1'-0". The actual embedment depth may vary plus or minus 6" along the length of the wall.
- 5. Post Spacing in this Index are nominal, and are measured from centerline to centerline of the auger cast piles. Actual post spacing may vary as shown in the Wall Control Drawings.
- 6. Panels:
 - A. The sum of the individual stacked panel heights is the Wall Height plus 1'-0" (embedment depth).
 - B. Where special graphics are required, locate the horizontal panel joints outside of the graphics. Where possible, hold horizontal panel joints at a constant elevation.
 - C. Side Installed Panels are only permitted when reduced overhead clearance between posts prohibits installing panels from the top.
 - 1. For Flush Face panels, install panel into posts from the roadway (front face) of the wall. Recessed panels may be installed from the back face of the wall.
 - 2. After panels are installed and centered between posts, grout between both panel ends and the adjoining posts (see Sheets 4 and 5 for details).
 - D. Individual panel heights should be between 6'-0" and 12'-0" tall. The minimum panel height is 4'-0" and may be used where overhead clearance is limited, or where graphic panels are required on shorter walls.
- 7. Concrete And Grout:
 - A. Concrete Class and Compressive Strength for:
 - 1. Precast Panels, Posts, and Post Caps: Class IV
 - 2. Cast-In-Place Collars: Class IV
 - B. Minimum Compressive Strength for form removal and handling of posts and panels:
 - 1. 2,500 psi for horizontally cast post and panels
 - 2,000 psi for vertically cast panels or when tilt-up tables are used for horizontally cast panels.
 - C. Grout for Auger Cast Piles:
 - 1. Maximum Working Compressive Strength = 2,000 psi
 - 2. Minimum 28 day strength = 5,000 psi
- 8. Reinforcing Steel:
 - A. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
 - 1. Post Stirrups Tie at all four corner bars and at every third interior bar intersection.
 - 2. Pile Stirrups Tie to the main vertical reinforcing at alternate intersections for circular configurations and at the four corners and at every third interior bar intersection for rectangular configurations.
 - B. Provide 2" concrete cover unless noted otherwise.
- 9. Casting Tolerances for precast panels and posts:
 - A. Overall Height and Width: $+/-\frac{1}{4}$ "
 - B. Thickness: $+/-\frac{1}{4}$ "
 - C. Plane of side mold: +/- 1/16"
 - D. Openings: +/- ½"

DESCRIPTION:

- E. Out of Square: 1/8" per 6 ft., but not more than 3/8"total along any side
- F. Warping: 1/16" per foot distance to nearest corner
- G. Bowing: 1/240 panel dimension
- H. Surface Smoothness for Type "A" Smooth Surface Texture Option: +/- 1/16"

10. Provide Plain or Fiber Reinforced Bearing Pads meeting the requirements of Specification Section 932 for Ancillary Structures.

2. Plain Pads may be substituted for Fiber Reinforced Pads when

sufficient bearing area is available on the concrete collar for the

b. 20' Post Spacing and Wall Height < 17 feet: 4"x 4"x \frac{1}{2}"

c. 20' Post Spacing and Wall Height ≥ 17 feet: 4"x 5"x ½"

B. At panel bearing points between stacked panels, use Plain or Fiber Reinforced

1. $4"x \ 4"x \ \frac{1}{2}"$ Fiber Reinforced Pads;

a. 10' Post Spacing: $4''x \ 4''x \ \frac{1}{2}''$

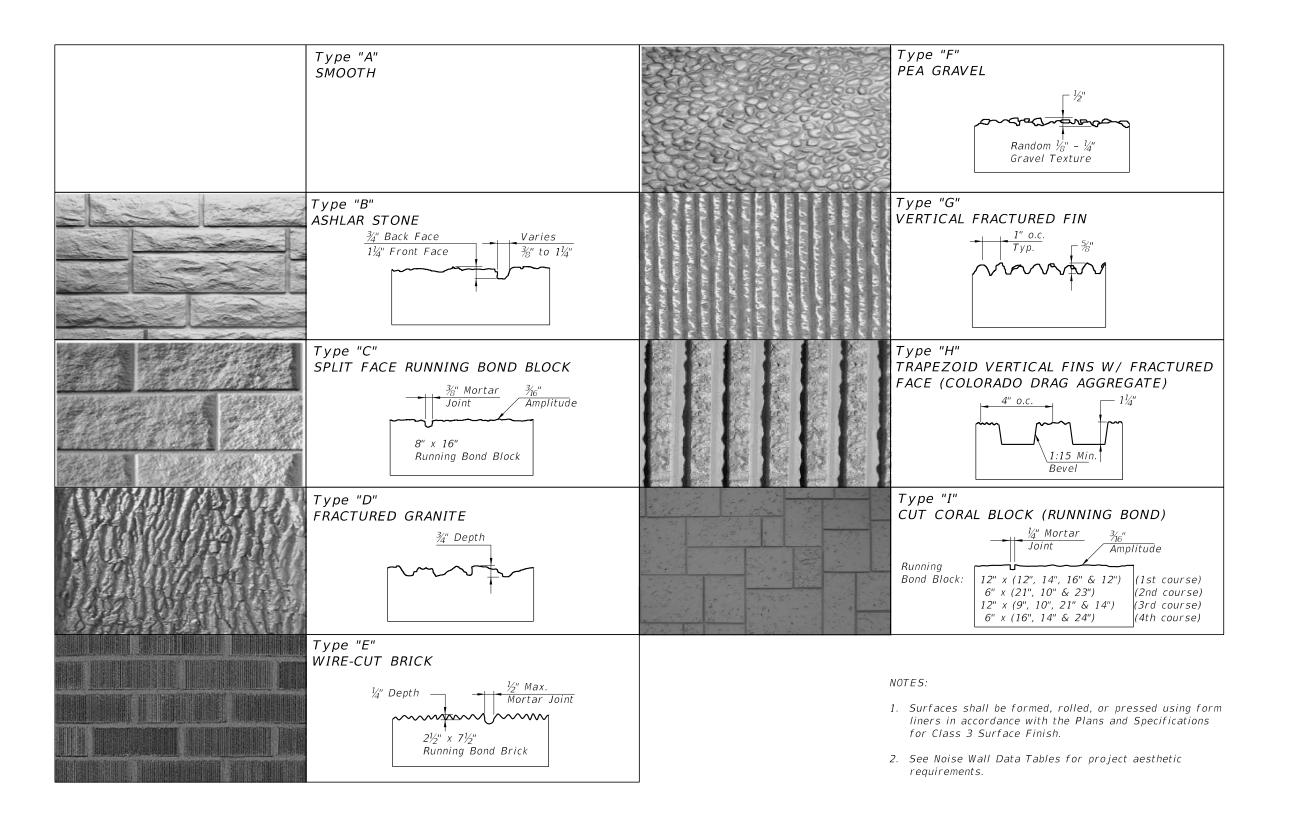
A. For Collar Bearing Points provide:

Bearing Pads.

INDEX SHEET

GENERAL NOTES

10/25/2017



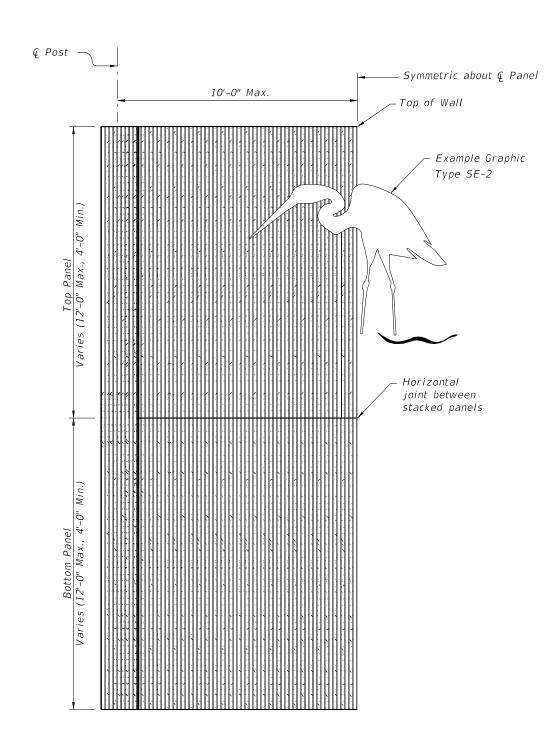
TEXTURE OPTIONS

REVISION 07/01/13

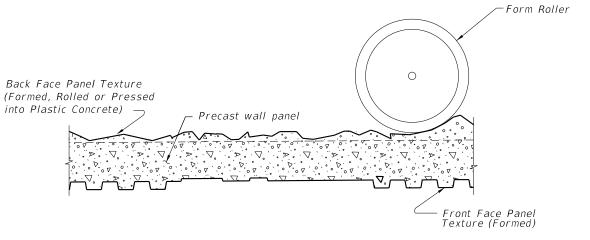
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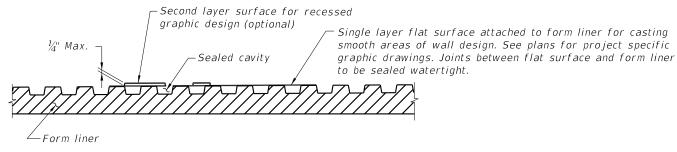
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FY 2018-19 STANDARD PLANS



HALF ELEVATION (Front Face Post and Panel Texture Type "H" shown) (Graphic Type SE-2 shown) (Two stacked panels shown, three stacked panels similar)





TYPICAL FORMING DETAIL (Front Face Panel Texture Type "H" shown) (Back Face Panel Texture Type "D" shown) (Post Forming Details Similar)

NOTES:

- 1. Submit specific form liner samples for approval by the Engineer.
- 2. Textures and graphics shown are for demonstration purposes only. See Noise Wall Data Tables in the plans for project specific texture and graphic requirements.

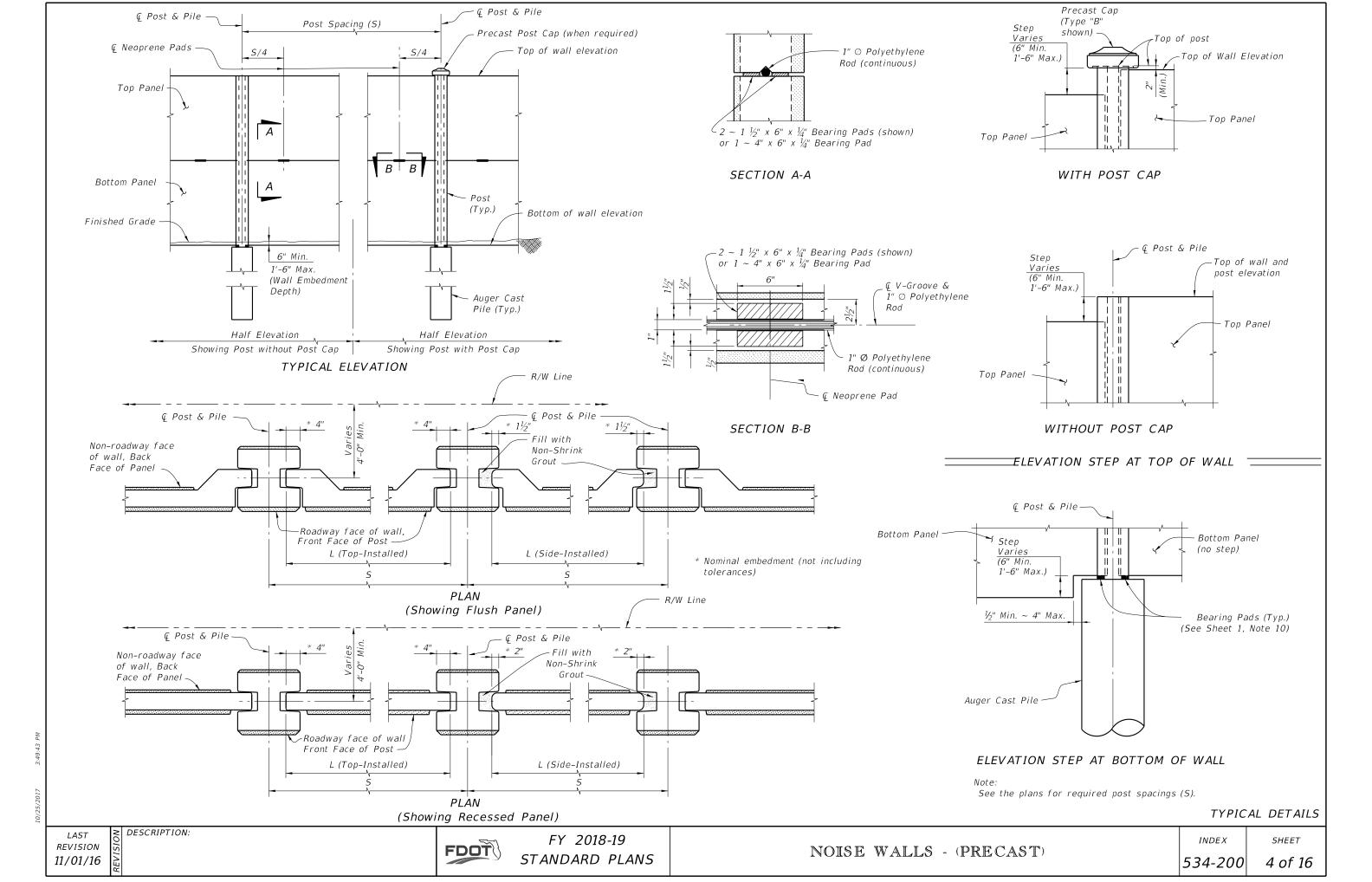
GRAPHICS & TEXTURE DETAILS

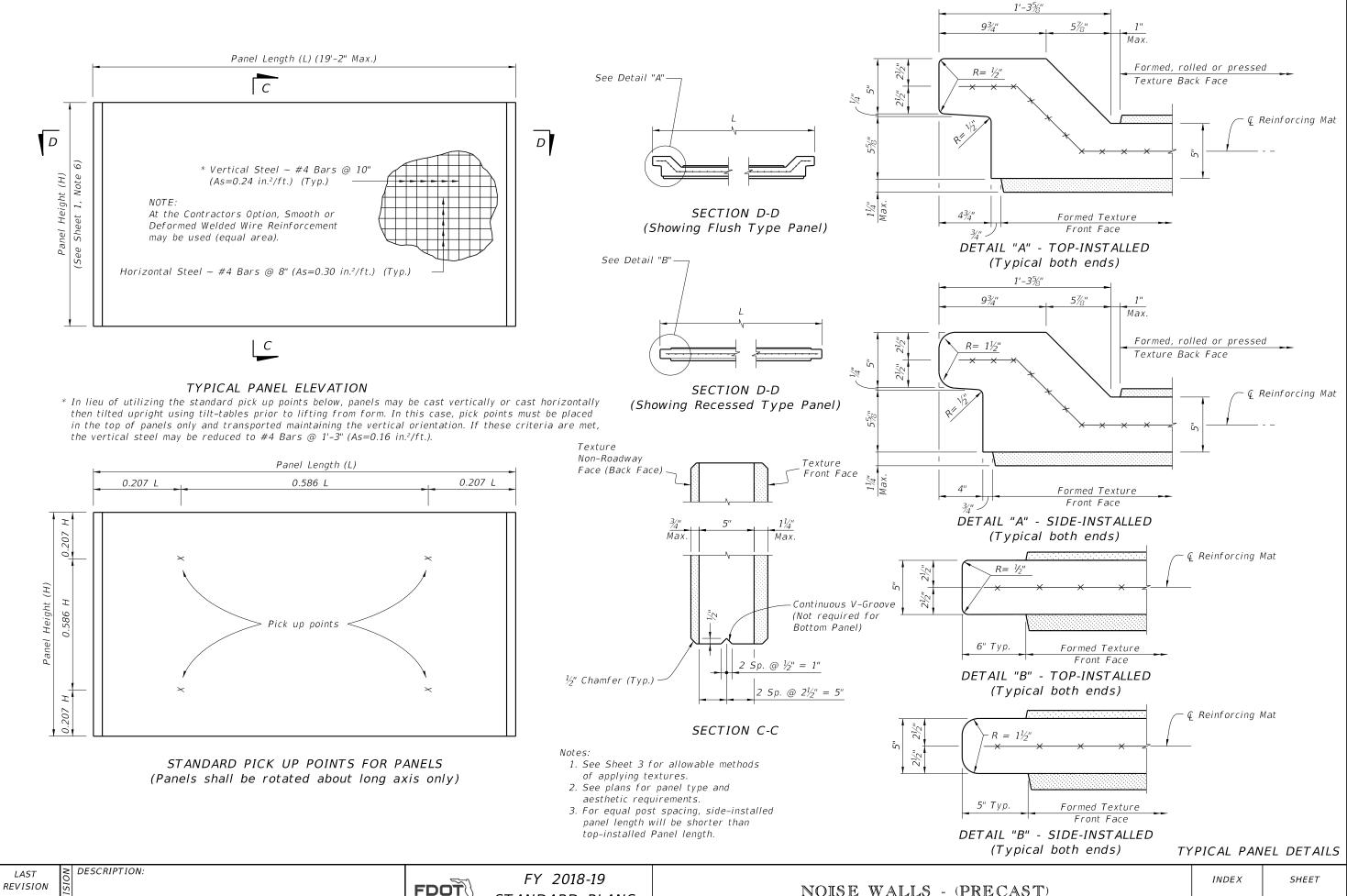
REVISION 07/01/14

DESCRIPTION:

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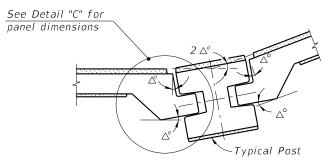
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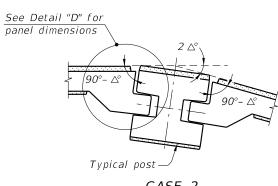
STANDARD PLANS

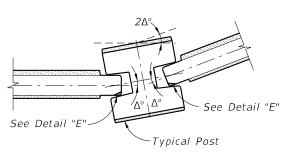
NOISE WALLS - (PRECAST)

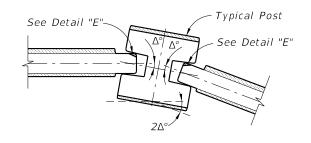
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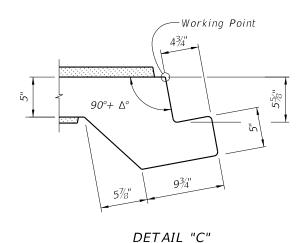


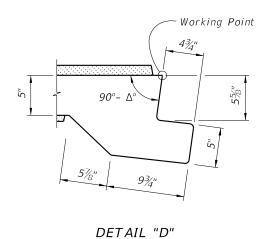


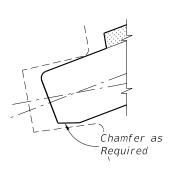
CASE 1 (Interior Angle)

CASE 2 (Exterior Angle)

CASE 1 CASE 2 (Exterior Angle) (Interior Angle)







DETAIL "E" (Back Face Chamfer Shown Front Face Chamfer Similar)

The shop drawings shall include specific pivoting details of panel ends at locations where the deflection angle (2 Δ °) between panels exceeds 7°.

The shop drawings shall include specific pivoting details of panel ends at locations where the deflection angle ($2\Delta^{\circ}$) between panels exceeds 20°.

PIVOTING DETAILS -(Flush Type Panel)

- PIVOTING DETAILS -(Recessed Type Panel)

TYPICAL PANEL DETAILS

REVISION 07/01/13

DESCRIPTION:

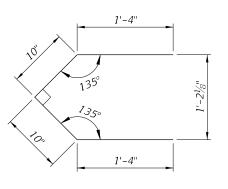
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NOISE WALLS - (PRECAST)

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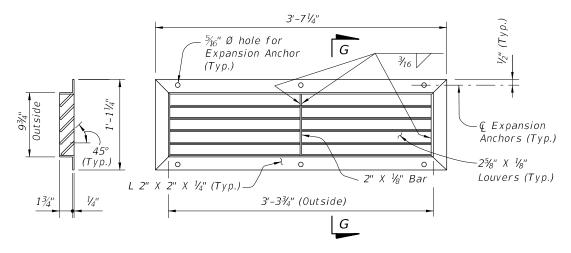
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DRAINAGE HOLES TYPES A, B, C & D (Front Face of Wall Shown) (Two Holes Shown, One Hole Similar)

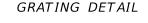


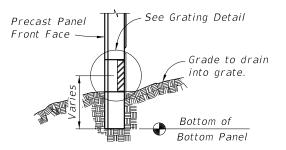
BAR A2 (Pair) Bar Length = 4'-4''

🗆 BAR BENDING DETAILS (#3 Bars) 💳



SECTION G-G





SECTION F-F

GRATING NOTES:

- 1. Grating shall be ASTM A36 steel welded in accordance with the current edition of ANSI/AWS D1.1 Steel Welding Code. Hot-dip galvanize grate after fabrication in accordance with Specification
- 2. Expansion Anchors: Use $\frac{1}{4}$ " Ø x 2" min. corrosion resistant (zinc/aluminum alloy or stainless steel) expansion anchors to connect grates to panels.
- 3. Blockout textured concrete surface for a strip 2" wide around drainage hole to enable secure attachment of the drainage grate.

DRAINAGE HOLE DETAILS

REVISION 11/01/17

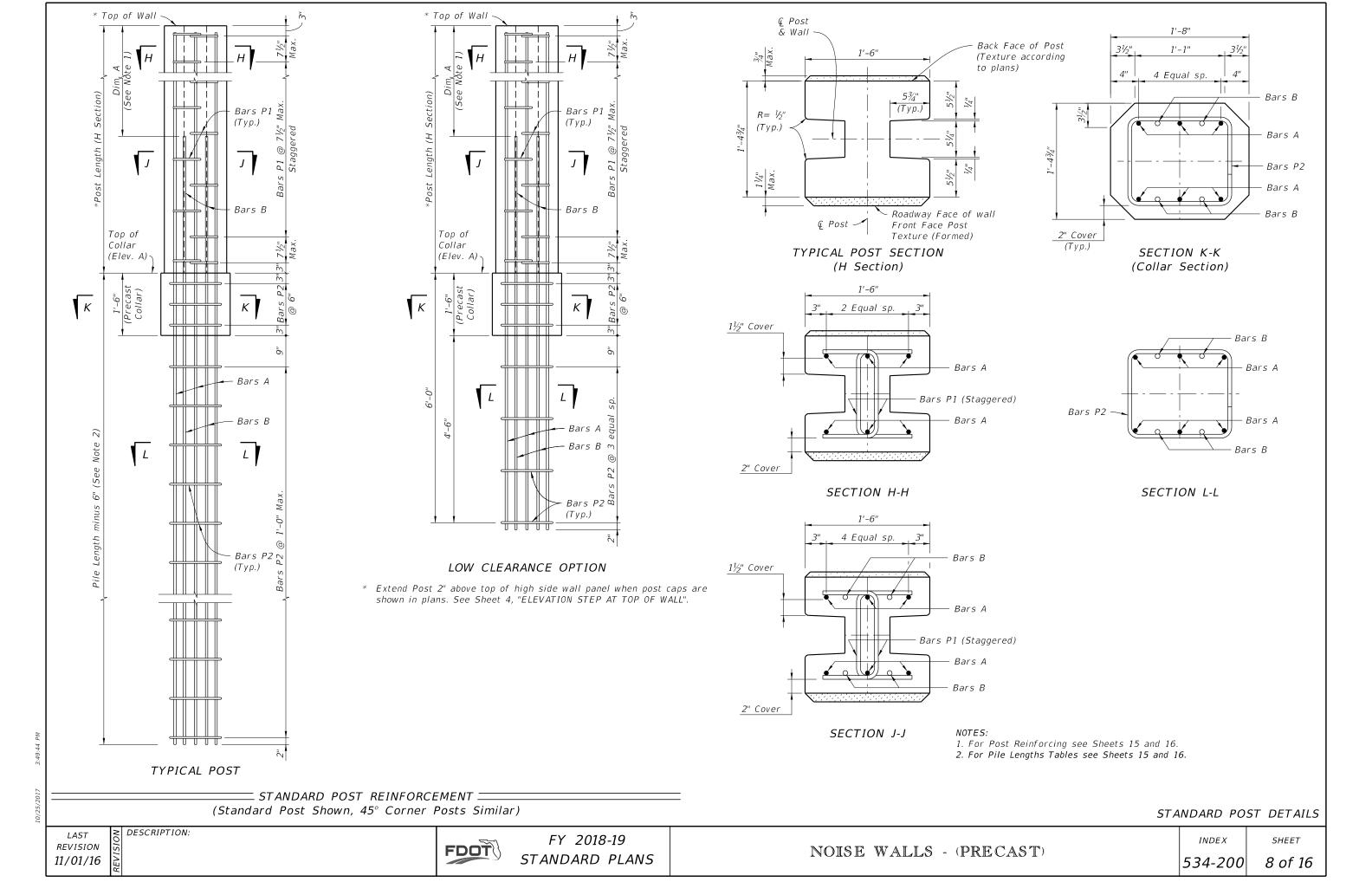
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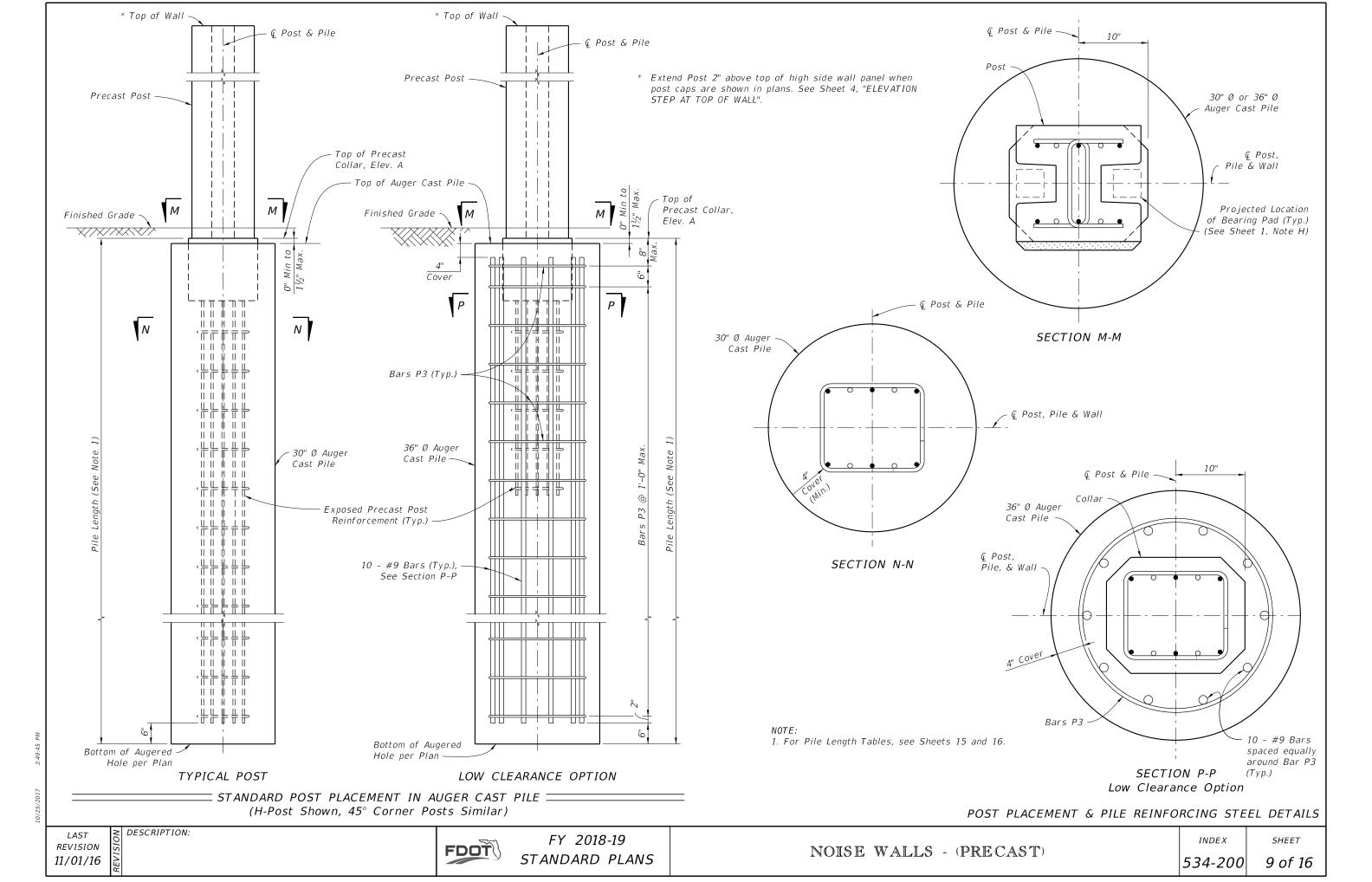
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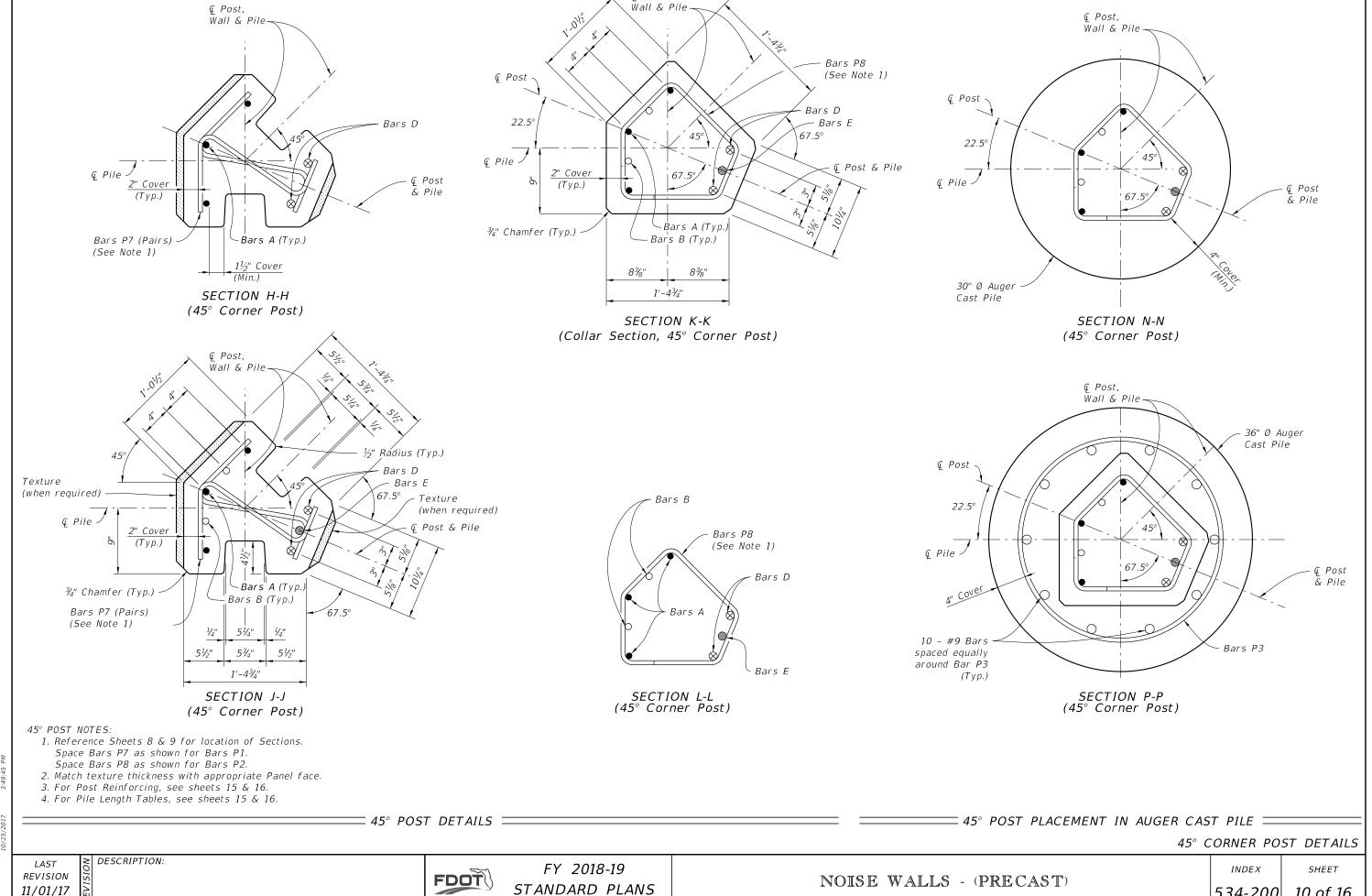
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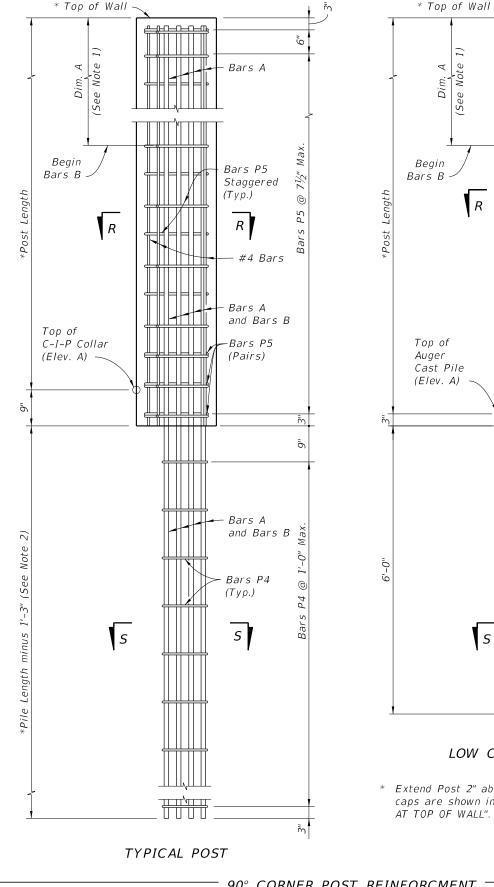


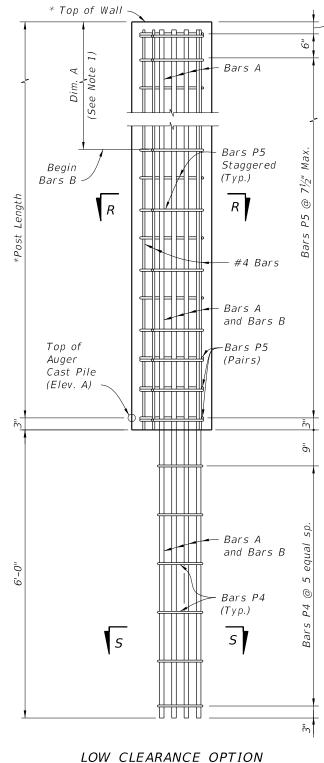


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€ Post,

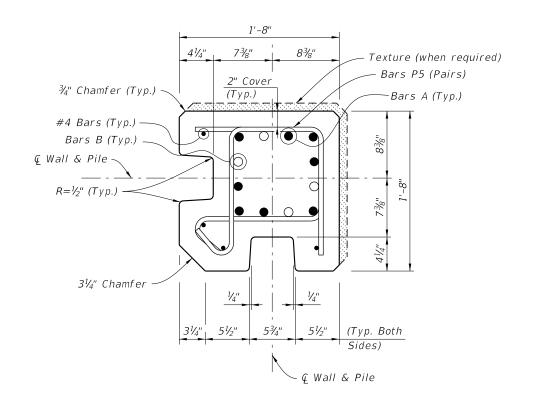
11/01/17



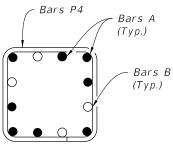


* Extend Post 2" above top of high side wall panel when post caps are shown in plans. See Sheet 4, "ELEVATION STEP

 \equiv 90 $^{\circ}$ CORNER POST REINFORCMENT \equiv (Post Surface Features Not Shown For Clarity)



SECTION R-R



SECTION S-S

90° CORNER POST NOTES:

- 1. For Post Reinforcing, see Sheets 15 and 16.
- 2. For Pile Length Tables, see Sheets 15 and 16.
- 3. Reduce typical panel length or adjust pile spacing at each 90° Corner Post.
- 4. Match texture thickness with appropriate Panel face.

90° CORNER POST DETAILS

REVISION 11/01/16

DESCRIPTION:

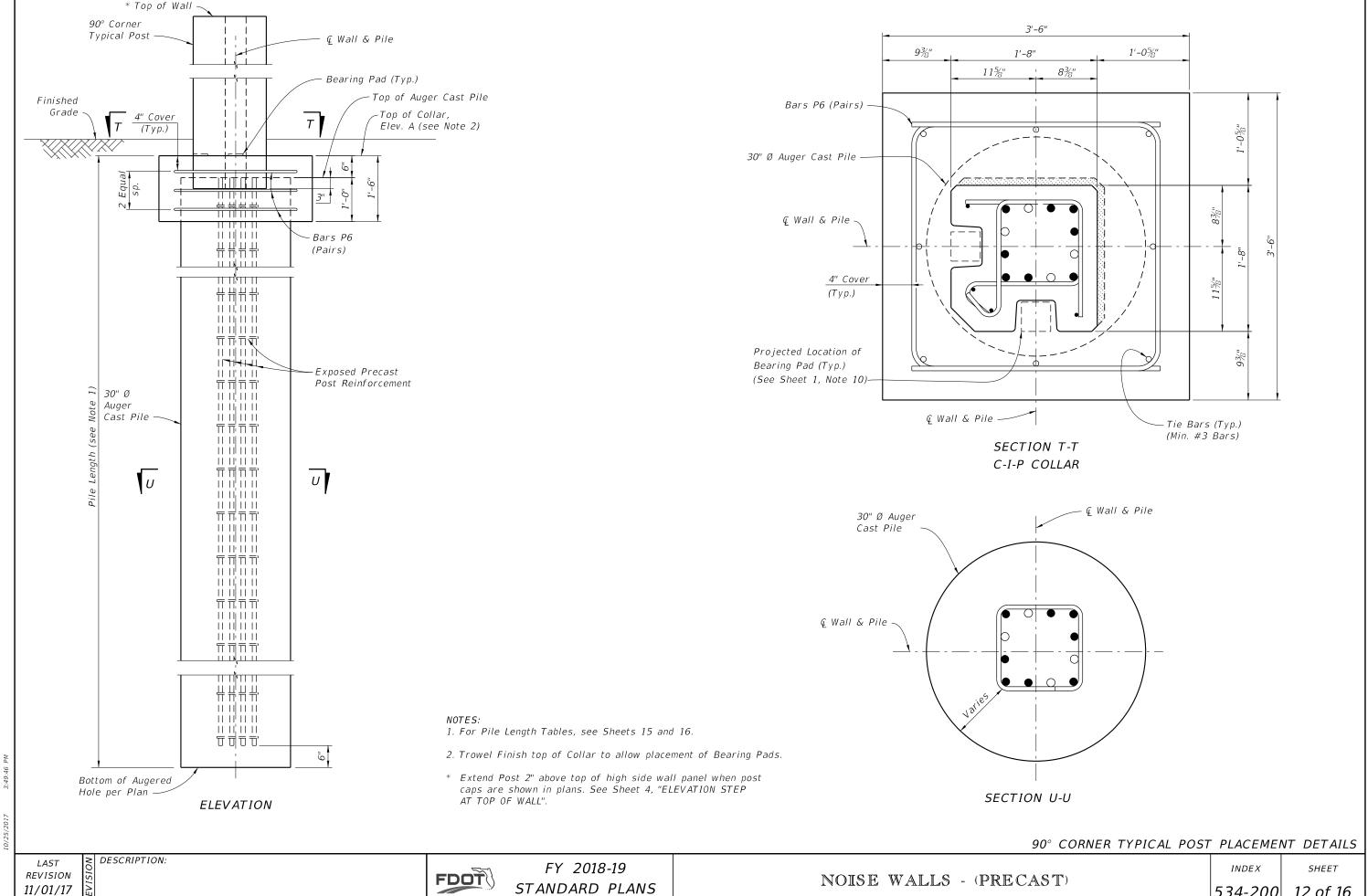
FDOT

FY 2018-19 STANDARD PLANS

NOISE WALLS - (PRECAST)

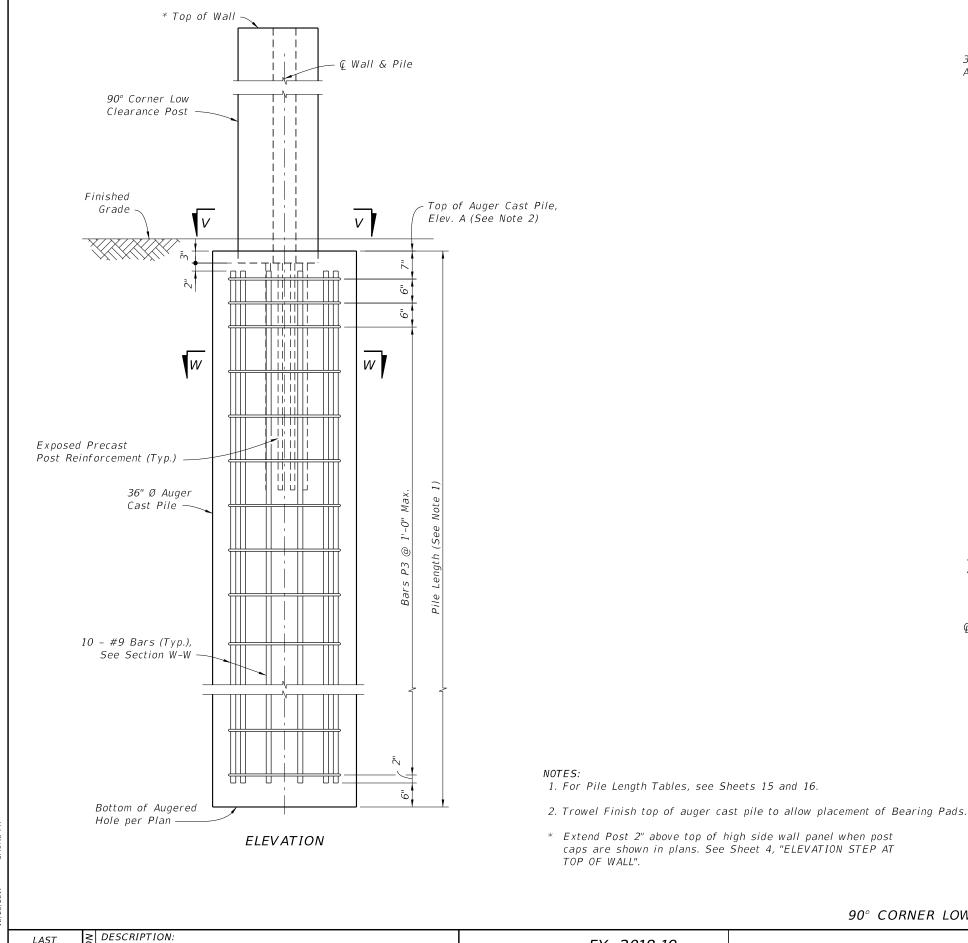
INDEX SHEET

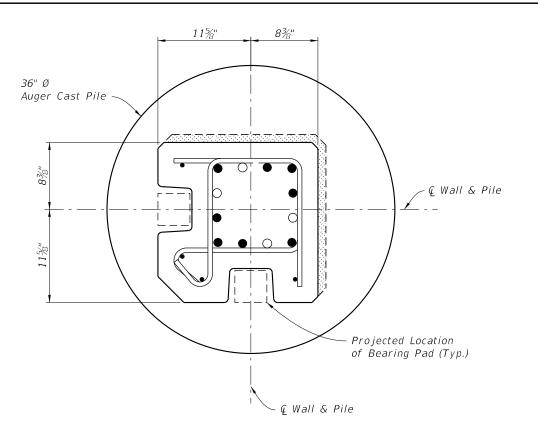
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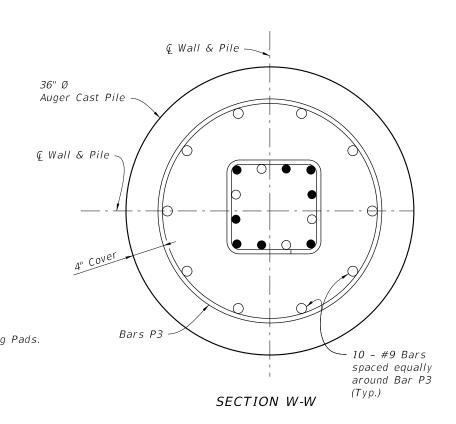
STANDARD PLANS

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SECTION V-V



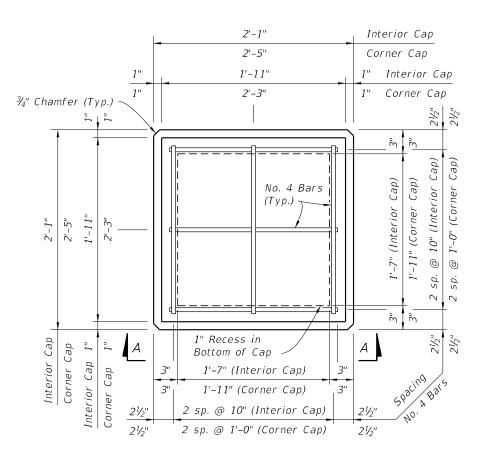
90° CORNER LOW CLEARANCE POST PLACEMENT & PILE REINFORCING STEEL DETAILS

FDOT

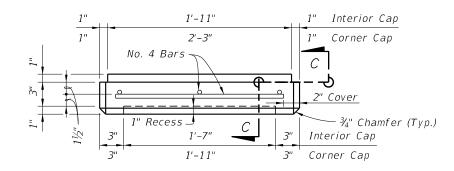
FY 2018-19 STANDARD PLANS

INDEX

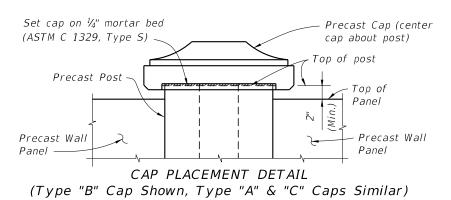
SHEET |534-200| 13 of 16

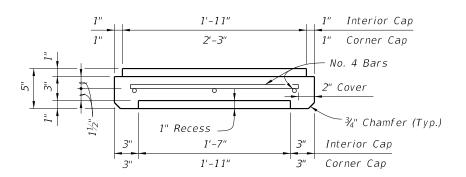


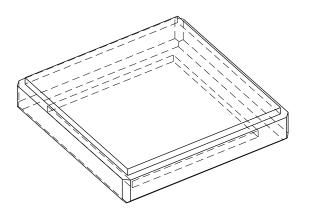
PLAN VIEW (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



VIEW A-A SHOWN, VIEW B-B SIMILAR (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



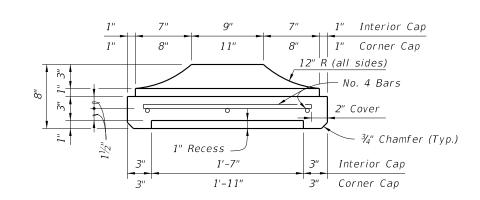


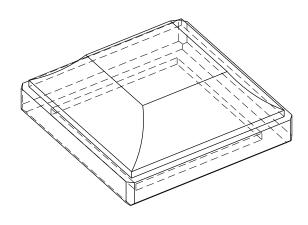


SECTION C-C

PICTORIAL VIEW

= TYPE "A" CAP DETAILS =

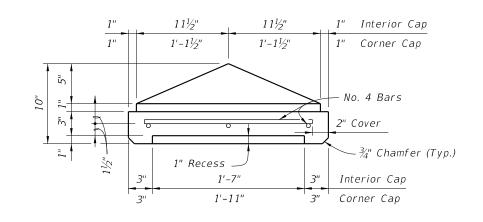


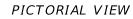


SECTION C-C

PICTORIAL VIEW

TYPE "B" CAP DETAILS =





SECTION C-C

= TYPE "C" CAP DETAILS ==

PRECAST POST CAPITAL

REVISION 07/01/14

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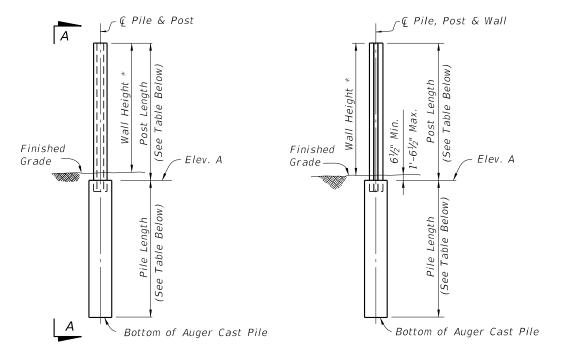
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FY 2018-19 STANDARD PLANS

NOISE WALLS - (PRECAST)

INDEX SHEET

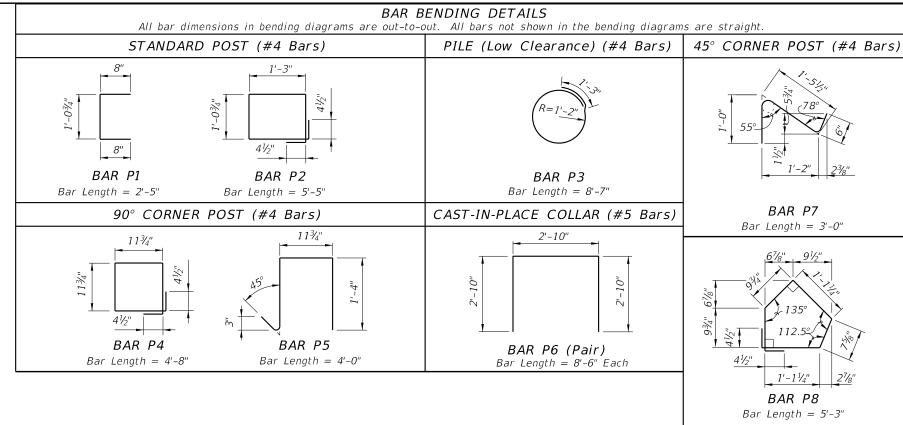
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* See Sheet 1, Note 4.

VIEW A-A

PILE/POST ELEVATION



		T	ABLE 1	'A - T.	ABLE (OF PO	ST RE	INFOR	CING S	STEEL								TAE	3LE 1B	- PILE	ELEN	GTHS ((Feet)	- WIN	D SPE	ED =	130 M	PH			
	POST LE	ENGTHS					WIND	SPEEL	0 = 13	0 MPH	ı					10'-0" POST SPACING 20'-0" POST SPACING															
NOMINAL WALL						'-0" SPACING						'-0" SPACING			NOMINAL WALL		H-P)STS			CORNER	R POSTS			H-P	0STS			CORNER	R POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	ВА	ARS B	BARS D	BA	ARS E	BARS A	BA	RS 3	BARS D	В	ARS E	HEIGHT (Feet)	50.	L 1	501	TL 2	501	IL 1	501	L 2	501	!L 1	50	IL 2	501	IL 1	50.	IL 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		<i>30</i> " ∅	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ∅	<i>30</i> " ⊘	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ∅
12	13'-0½"	13'-21/2"	#4	#4	7'-11"	#4	#4	9'-11"	#5	#5	9'-8"	#6	#6	9'-4"	12	11	10	10	10	11	10	10	10	15	14	13	12	14	13	13	12
13	14'-0½"	14'-2½"	#4	#4	10'-11"	#4	#4	10'-11"	#5	#5	9'-8"	#6	#6	9'-4"	13	12	11	10	10	11	10	10	10	15	14	13	13	15	14	13	12
14	15'-0 ¹ / ₂ "	15'-2 ¹ / ₂ "	#4	#4	10'-11"	#5	#5	11'-8"	#6	#6	11'-4"	#7	#7	10'-8"	14	12	11	11	10	12	11	10	10	16	15	14	13	15	14	14	13
15	16'-0 ¹ / ₂ "	16'-2 ¹ / ₂ "	#4	#4	10'-11"	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8"	15	12	12	11	10	12	11	11	10	16	15	15	13	16	15	14	13
16	17'-0 ¹ / ₂ "	17'-2½"	#5	#5	13'-8"	#5	#5	12'-8"	#6	#6	11'-4"	#7	#7	10'-8"	16	13	12	11	11	12	12	11	10	17	16	15	14	16	15	15	14
17	18'-0 ¹ / ₂ "	18'-2 ¹ / ₂ "	#5	#5	14'-8"	#5	#5	12'-8"	#7	#7	12'-8"	#7	#8	10'-0"	17	13	12	12	11	13	12	11	11	18	16	16	14	17	16	15	14
18	19'-0 ¹ / ₂ "	19'-2 ¹ / ₂ "	#5	#5	14'-8"	#6	#6	14'-4"	#7	#7	12'-8"	#8	#8	12'-0"	18	14	13	12	11	13	12	12	11	18	17	16	15	18	16	15	14
19	20'-0 ¹ / ₂ "	20'-2 ¹ / ₂ "	#5	#5	14'-8"	#6	#6	14'-4"	#7	#8	12'-0"	#8	#9	11'-3"	19	14	13	12	12	14	13	12	11	19	17	16	15	18	17	16	15
20	21'-0 ¹ / ₂ "	21'-2½"	#6	#6	16'-4"	#6	#6	14'-4"	#8	#7	14'-8"	#9	#8	14'-0"	20	14	13	13	12	14	13	12	12	19	18	17	16	19	17	16	15
21	22'-0 ¹ / ₂ "	22'-2 ¹ / ₂ "	#6	#6	16'-4"	#6	#6	14'-4"	#8	#8	14'-0"	#9	#10	12'-4"	21	15	14	13	12	14	13	13	12	20	18	17	16	19	18	17	16
22	23'-01/2"	23'-2 ¹ / ₂ "	#6	#6	16'-4"	#7	#7	16'-8"	#8	#9	13'-3"	#10	#9	15'-3"	22	15	14	14	13	15	14	13	12	20	19	18	17	20	18	17	16

TABLE NOTE:

- 1. Bars D and Bars E are for 45° Corner Posts only.
- 2. See Contract Plans for project wind speed.
- 3. Soil 1 = Loose Granular Soil, N = 4 to 9.
 - Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

PILE DEPTH & REINFORCING SUMMARY

REVISION 11/01/16

DESCRIPTION:

FDOT

FY 2018-19 STANDARD PLANS

NOISE WALLS - (PRECAST)

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		Τ	ABLE 3	8A - T.	ABLE (OF PO	ST RE	INFOR	CING	STEEL								TAE	BLE 3B	B - PIL	E LEN	GTHS	(Feet)	- WIN	ID SPI	EED =	170 M	IРН			
	POST LI	ENGTHS					WIND	SPEEL) = 17	O MPH	1					10'-0" POST SPACING 20'-0" POST SPACING															
NOMINAL WALL						"-0" SPACING						'-0" SPACING			NOMINAL WALL		H-P(OSTS			CORNE	R POSTS			H-P()STS			CORNEF	R POSTS	
HEIGHT (Feet)	WITHOUT CAP	WITH CAP	BARS A	BA	iRS B	BARS D	BA	ARS E	BARS A	BA	ARS B	BARS D	В	ARS E	HEIGHT (Feet)	50.	L 1	50.	IL 2	50	IL 1	501	L 2	501	L 1	501	L 2	50	IL 1	501	IL 2
			SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'	SIZE	SIZE	DIM 'A'		<i>30</i> " ⊘	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ∅	<i>30</i> " ∅	36" ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘	30" ⊘	<i>36</i> " ⊘	<i>30</i> " ⊘	<i>36</i> " ⊘
12	13'-0 ¹ / ₂ "	13'-2 ¹ / ₂ "	#5	#5	9'-8"	#5	#5	8'-8"	#6	#6	8'-4"	#7	#7	7'-8"	12	14	13	12	11	13	12	12	11	18	17	16	15	18	16	16	15
13	14'-0½"	14'-2 ¹ / ₂ "	#5	#5	10'-8"	#6	#6	10'-4"	#7	#7	8'-8"	#8	#7	8'-8"	13	14	13	13	12	14	13	12	11	19	18	17	16	19	17	16	15
14	15'-0½"	15'-2 ¹ / ₂ "	#5	#5	10'-8"	#6	#6	10'-4"	#7	#7	8'-8"	#8	#8	8'-0"	14	15	14	13	12	14	13	13	12	20	18	18	16	19	18	17	16
15	16'-0 ¹ / ₂ "	16'-2½''	#6	#6	12'-4"	#6	#6	10'-4"	#8	#7	10'-8"	#9	#8	10'-0"	15	15	14	14	13	15	14	13	12	21	19	18	17	20	18	18	16
16	17'-0½"	17'-2½"	#6	#6	12'-4"	#7	#7	11'-8"	#8	#8	10'-0"	#9	#10	8'-4"	16	16	15	14	13	15	14	14	13	21	20	19	17	21	19	18	17
17	18'-0 ¹ / ₂ "	18'-2 ¹ / ₂ "	#6	#6	12'-4"	#7	#7	11'-8"	#9	#8	12'-0"	#10	#9	10'-3"	17	16	15	15	14	16	15	14	13	22	20	19	18	21	20	19	17
18	19'-0 ¹ / ₂ ''	19'-2½"	#7	#7	13'-8"	#7	#8	11'-0"	#9	#10	10'-4"	#10	#11	8'-5"	18	17	16	15	14	16	15	15	14	23	21	20	19	22	20	19	18
19	20'-0 ¹ / ₂ "	20'-2 ¹ / ₂ "	#7	#7	13'-8"	#8	#7	13'-8"	#10	#10	11'-4"	#11	#11	10'-5"	19	17	16	15	14	17	16	15	14	23	22	21	19	23	21	20	18
20	21'-0 ¹ / ₂ "	21'-2 ¹ / ₂ "	#7	#7	13'-8"	#8	#8	13'-0"	#10	#11	10'-5"	#11	#14	7'-0"	20	18	17	16	15	17	16	15	14	24	22	21	20	23	21	20	19
21	22'-0 ¹ / ₂ "	22'-2 ¹ / ₂ "	#7	#8	13'-0"	#9	#8	15'-0"	#11	#10	13'-4"	#14	#11	12'-5"	21	18	17	16	15	18	17	16	15	25	23	22	20	24	22	21	19
22	23'-0 ¹ / ₂ "	23'-2 ¹ / ₂ "	#8	#7	16'-8"	#9	#9	14'-3"	#11	#11	12'-5"	#14	#14	9'-0"	22	19	18	17	16	18	17	16	15	25	23	22	21	24	23	22	20

TABLE NOTE:

- 1. Bars D and Bars E are for 45° Corner Posts only.
- 2. See Contract Plans for project wind speed.
- 3. Soil 1 = Loose Granular Soil, N = 4 to 9;

DESCRIPTION:

Soil 2 = Medium Dense Granular Soil, N = 10 to 40.

PILE DEPTH & REINFORCING SUMMARY

LAST REVISION

FDOT

FY 2018-19 STANDARD PLANS

NOISE WALLS - (PRECAST)

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GENERAL NOTES:

- 1. Construct Perimeter Walls in accordance with Specification Section 534.
- 2. Choice of either Precast Option or Masonry Option is at the discretion of the Contractor. Contractor must also select the desired foundation type. Modifications to this Index is restricted to those required for geometric needs only.
- 3. Post spacing is measured from centerline to centerline of foundation element. For this Index, posts and foundation elements have been designed for 20 ft. spacings. Use post spacings less than 20 feet only at changes in horizontal alignment, wall terminations or to accommodate
- 4. See "Perimeter Wall Data Tables" in the plans for project requirements.
- 5. Field verify the locations of all overhead and underground utilities shown in the Wall Control

PRECAST OPTION NOTES:

6. WALL NOTES:

- A. Walls may consist of either a single height panel or two stacked panels. Minimum panel height is 4'-3".
- B. Only when reduced overhead clearance between posts prohibits installation of panels from the top, side-installed panels are allowed. After panel is centered between posts, grout between panel ends and posts.

7. CONCRETE AND GROUT:

- A. Cast-in-Place and Precast Concrete: Class IV
- B. Grout for Auger Cast Piling: Minimum 28 Day Strength = 5000 psi
- C. Minimum Compressive Strength for Form Removal and Handling of Posts, Panels and Precast Spread Footings:
 - i. 2,500 psi for horizontally cast post, panels and precast spread footings.
- ii. 2,000 psi for vertically cast panels or when tilt-up form tables are used for horizontally cast panels.

8. REINFORCING STEEL:

- A. Concrete Cover: $1\frac{1}{2}$ " unless otherwise noted.
- B. In addition to the requirements of Specification Section 415, tie post and pile stirrups at the following locations as a minimum:
- i. Post Stirrups Tie at all four corner bars and at every third interior bar intersection.
- ii. Pile Stirrups Tie to the main vertical reinforcing at alternate intersections.

9. BEARING PADS:

A. Bearing Pads for Collar or Pedestal Bearing Points and between stacked panels may be either Plain or Fiber Reinforced Neoprene Pads, in accordance with Specification Section 932 for ancillary structures.

10. CASTING TOLERANCES:

- A. Overall Height & Width: $+/-\frac{1}{4}$ "
- B. Thickness: $\pm 1/-\frac{1}{2}$ "
- C. Plane of side mold: $\pm \frac{1}{16}$
- D. Openings: $+/-\frac{1}{2}$ "
- E. Out of Square: $\frac{1}{8}$ " per 6 ft., but not more than $\frac{3}{8}$ " total along any side
- F. Warping: $\frac{1}{16}$ " per foot distance to nearest corner
- G. Bowing: 1/240 panel dimension

11. PILING:

Construct Auger Cast Piling in accordance with the Plans and Specification Section 455.

MASONRY OPTION NOTES:

DESCRIPTION:

12. WALL NOTES:

- A. Inspect construction in accordance with the International Building Code (IBC) Section 17.
- B. Construct masonry walls with 8x8x16 block using a running bond pattern and concave tooled joints.
- C. Make all elevation changes (steps) in footing and top of wall using full height blocks. Make top of wall steps at pilasters exclusively. Footing steps may be made between pilasters as necessary to maintain minimum soil cover.

MASONRY OPTION NOTES (CONT.):

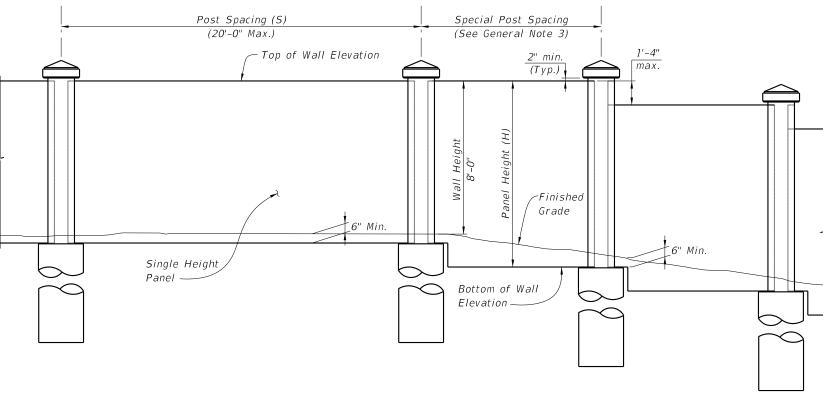
- D. Fully Grout all cells with horizontal or vertical reinforcing bars.
- E. Use reinforcing bar positioners to maintain vertical and horizontal bar placement.
- F. Fully grout first three courses of the wall.
- G. Joint Reinforcement: Use W 1.7 (9mm) galvanized ladder reinforcing spaced at 16" vertically. Provide special accessories for corners, intersections, etc. Joint reinforcing shall be continuous except it shall not pass through vertical masonry control joints. Lap joint reinforcing a minimum of 6".
- H. Construct expansion joints in the foundation at 90 foot maximum intervals, and directly below a wall control joint.
- I. Dowel Load Transfer Devices will be ASTM A 36 smooth round bars hot-dip galvanized in accordance with Specification Section 962. Install Dowel Load Transfer Devices in accordance with Specification Section 350.
- J. For spread footings, use a walk-behind compactor of at least 600 lbs. in weight. Obtain a minimum density of 95% of the maximum dry density as determined by FM 1 T-180. Perform soil density tests at 100 foot intervals.
- K. Protect walls during construction from soil, grout or mortar stains. Clean wall as work progresses by dry brushing to remove mortar fins and smears before tooling joints.
- L. Use soap and potable water to clean walls. If stain removal is necessary, use a cleaning method indicated in NCMA TEK 8-2A applicable to the type of stain on the exposed surface.
- M. During construction, cover tops of walls, with waterproof sheeting at the end of each day's work, or when construction is not in progress. Extend sheeting a minimum of 2 feet down each side and secure in place.
- N. Comply with Hot Weather Requirements in ACI 530.1.

13. MATERIALS:

- A. Concrete Masonry Units (CMU): Provide normal weight blocks.
- B. Cast-In-Place Concrete: Class II for slightly to moderate aggressive environments or Class IV for extremely aggressive
- C. Mortar: Type S meeting requirements of ASTM C1329
- D. Grout: Type S; coarse grout.
- E. Aggregate for Grout: Meet the requirements of ASTM C404 or Specification Section 901 size 8 or 89.

14. STORAGE OF MATERIALS:

- A. Store CMU's on elevated platforms in a dry location or under cover.
- If units become wet, do not install until they are dry.
- B. Store cementitious materials on elevated platforms, under cover, and in a dry location. Do not use cementitious materials that have become damp or exceded the manufacturers shelf life.
- C. Store masonry accessories and reinforcing to prevent corrosion and accumulation of dirt and oil.



GENERAL WALL ELEVATION (Precast Option with SIngle Height Panel Shown, Others Similar)

GENERAL NOTES

REVISION 11/01/17

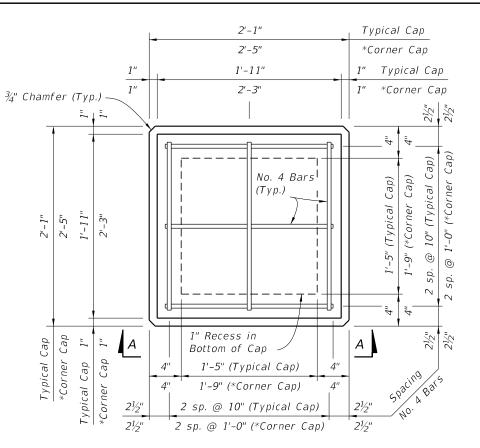
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FY 2018-19 STANDARD PLANS

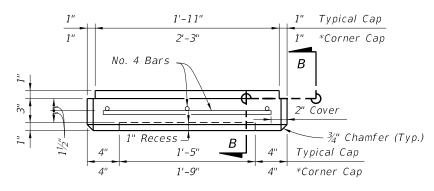
PERIMETER WALLS

INDEX 534-250

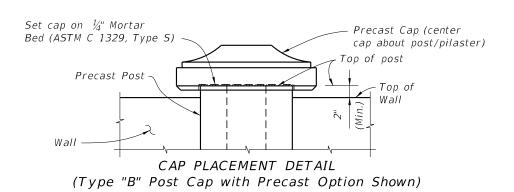
SHEET 1 of 10

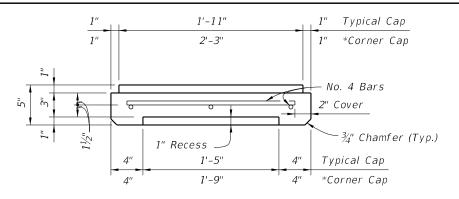


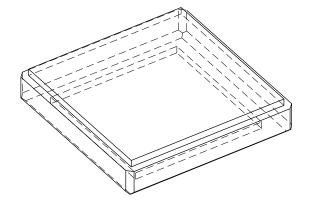
PLAN VIEW (Type "A" Cap Shown, Type "B" & "C" Caps Similar)



VIEW A-A (Type "A" Cap Shown, Type "B" & "C" Caps Similar)





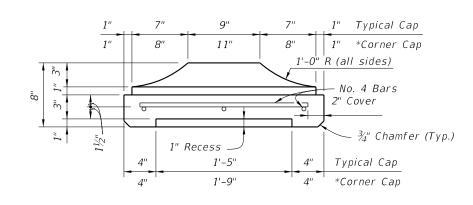


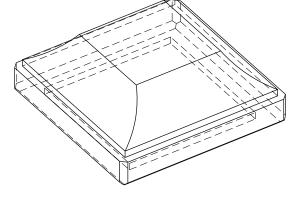
SECTION B-B

PICTORIAL VIEW

= TYPE "A" CAP DETAILS =

*Precast Option only

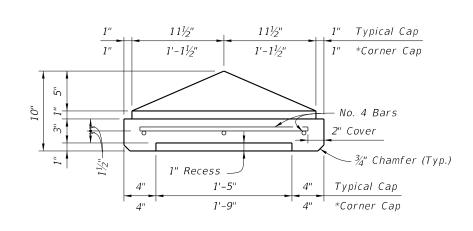


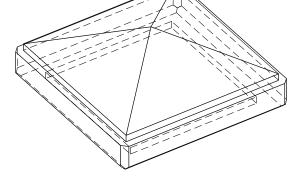


SECTION B-B

PICTORIAL VIEW

TYPE "B" CAP DETAILS





SECTION B-B

PICTORIAL VIEW

= TYPE "C" CAP DETAILS =

POST CAP DETAILS

REVISION 01/01/14

FDOT

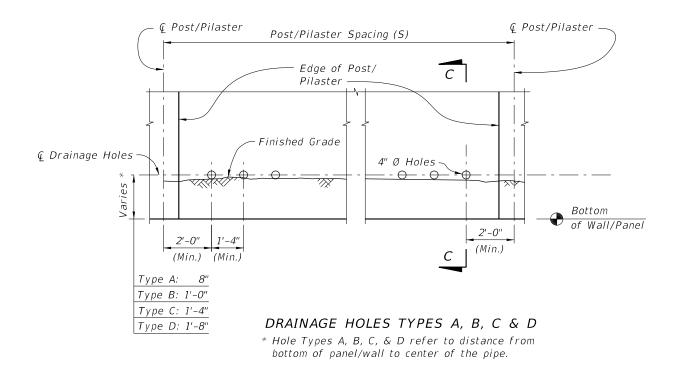
FY 2018-19 STANDARD PLANS

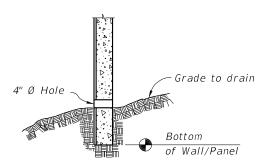
PERIMETER WALLS

INDEX 534-250

SHEET 2 of 10

DESCRIPTION:





SECTION C-C (Precast Option Shown, Masonry Option Similar)

- 1. Drainage holes may be formed with 4" NPS PVC pipe that may remain in place.
- 2. See Wall Control drawings for number, Type and location/ spacing of drainage holes.

DRAINAGE DETAILS

REVISION 01/01/14

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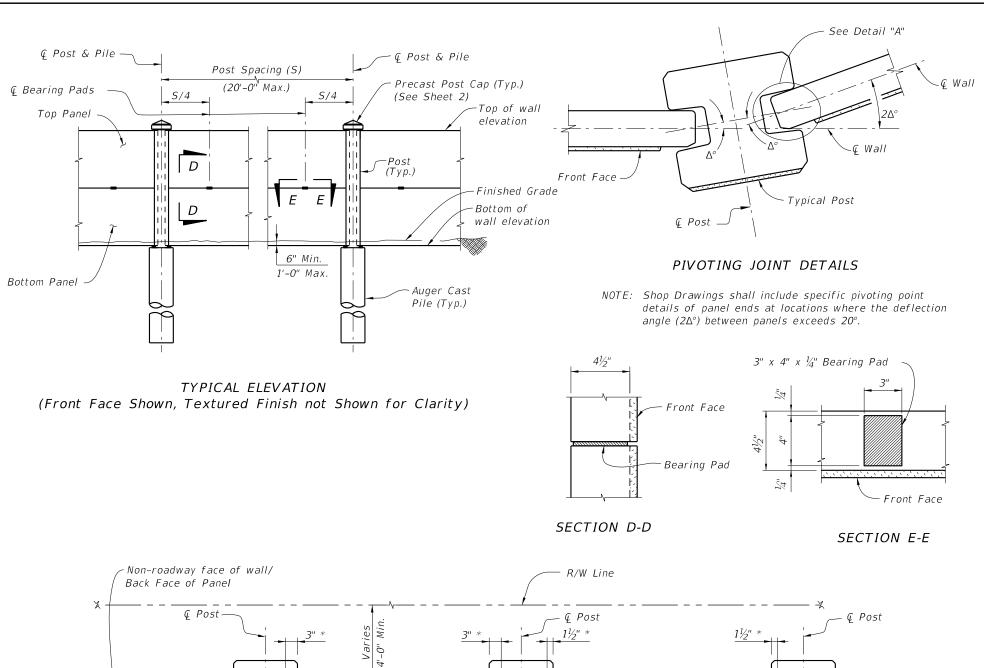
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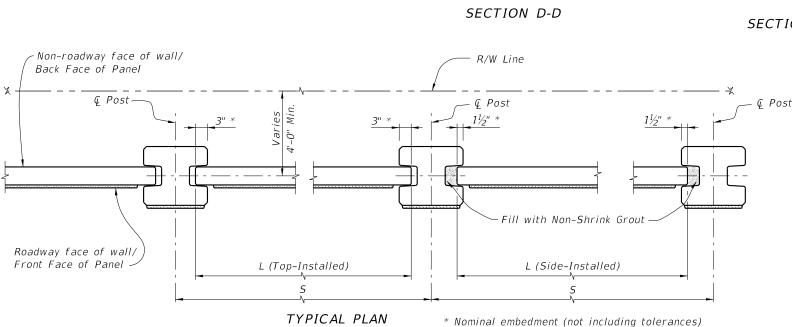
FY 2018-19 STANDARD PLANS

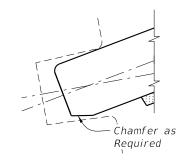
PERIMETER WALLS

INDEX

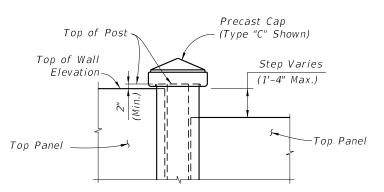
SHEET 534-250 3 of 10



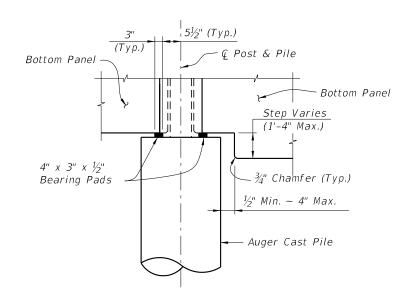




DETAIL "A" (Back Face Chamfer Shown Front Face Chamfer Similar)



ELEVATION STEP AT TOP OF WALL (Precast Panel Cap not Shown)



ELEVATION STEP AT BOTTOM OF WALL

PRECAST OPTION - TYPICAL DETAILS

REVISION 11/01/17

DESCRIPTION:

FDOT

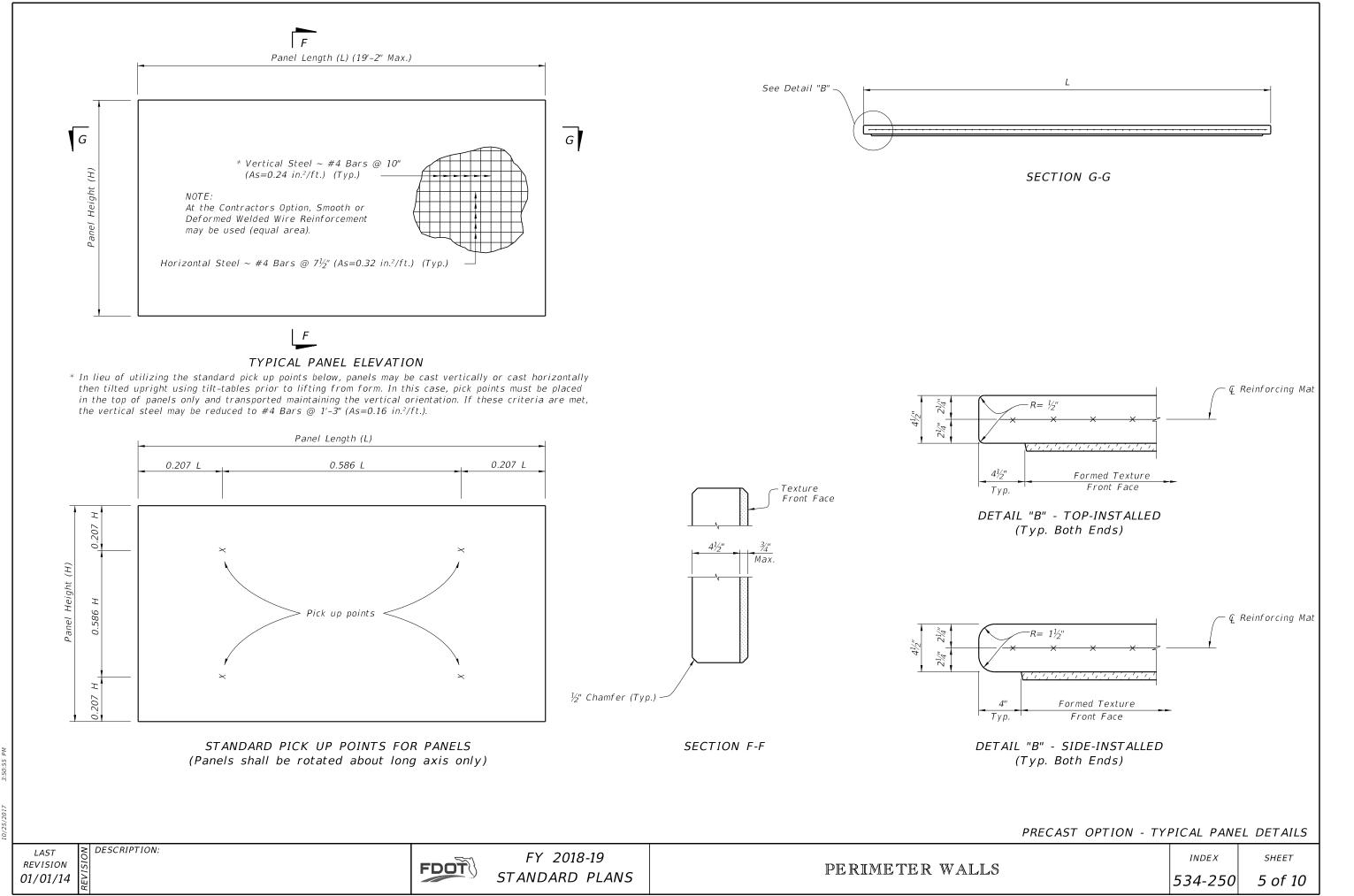
FY 2018-19 STANDARD PLANS

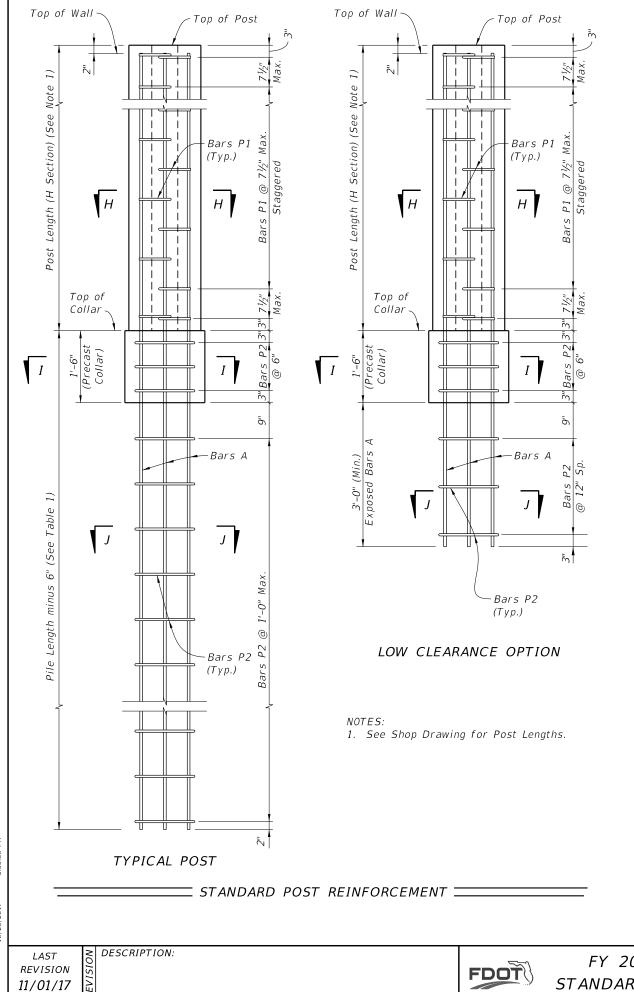
PERIMETER WALLS

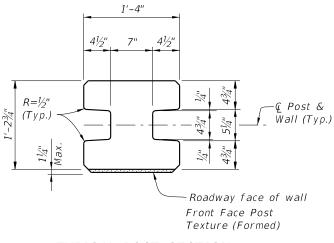
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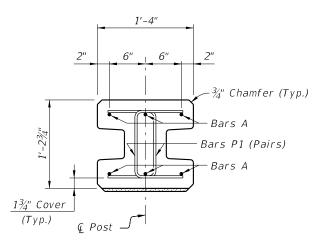
534-250





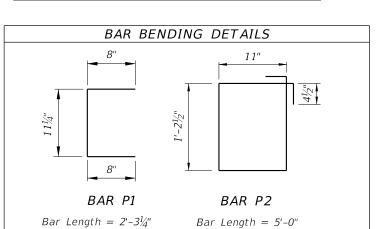


TYPICAL POST SECTION (H Section)



SECTION H-H (H Section - Above Collar)

Wind Speed (MPH)	Pile Length	Bars A	Bars P1 thru P6	Bars S1
130	12'-0"	#5	#3	#4
150	13'-6"	#5	#3	#4
170	15'-0"	#6	#3	#4



All bar dimensions in bending diagrams are out-to-out.

All bars not shown in the bending diagrams are straight.

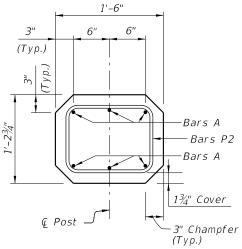
SECTION I-I Precast Collar

¾" chamfer (Typ.) —

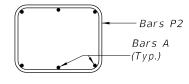
1'-6"

_ Bars A

Bars P2



SECTION I-I (for Low Clearance Option)

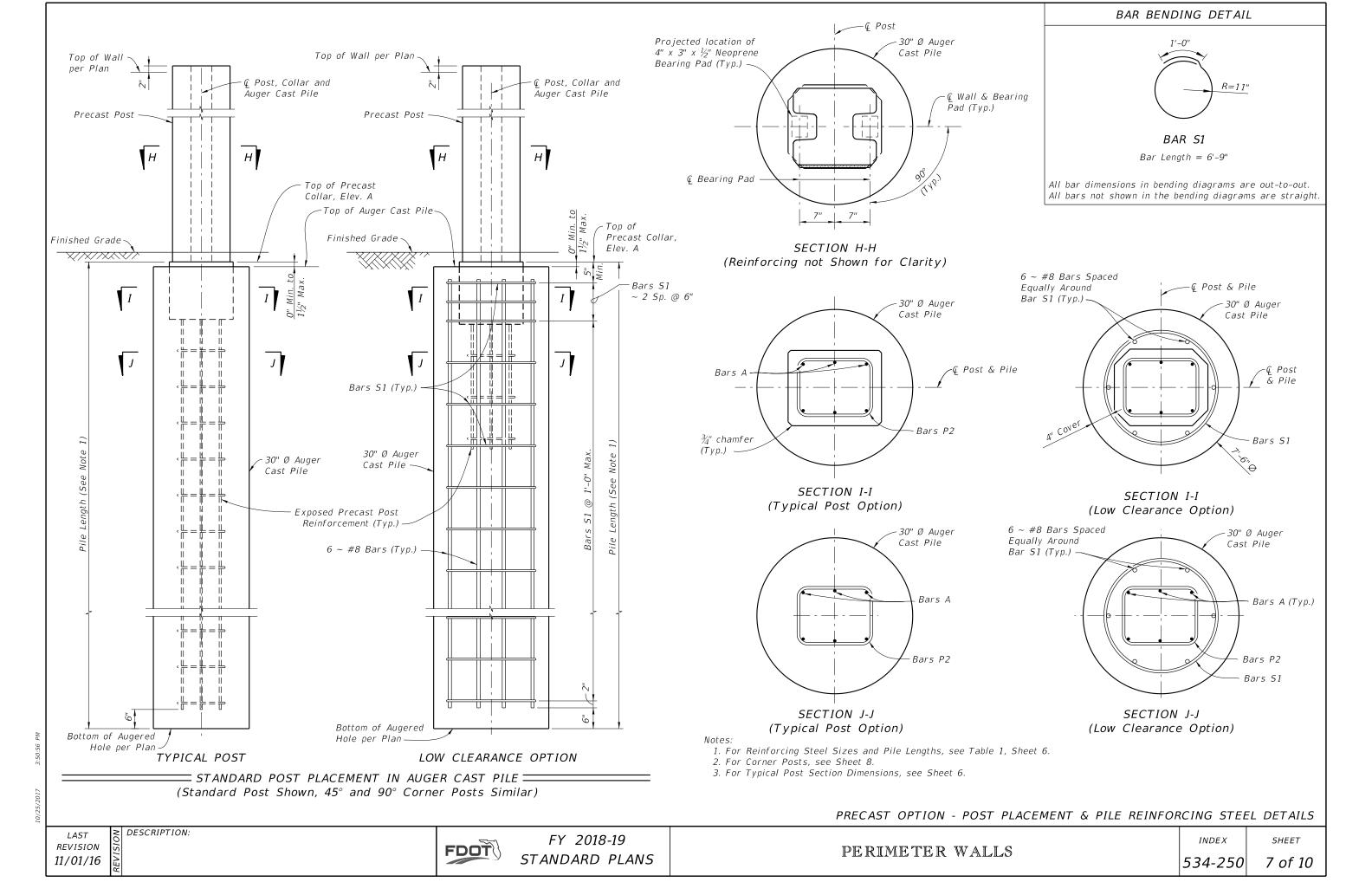


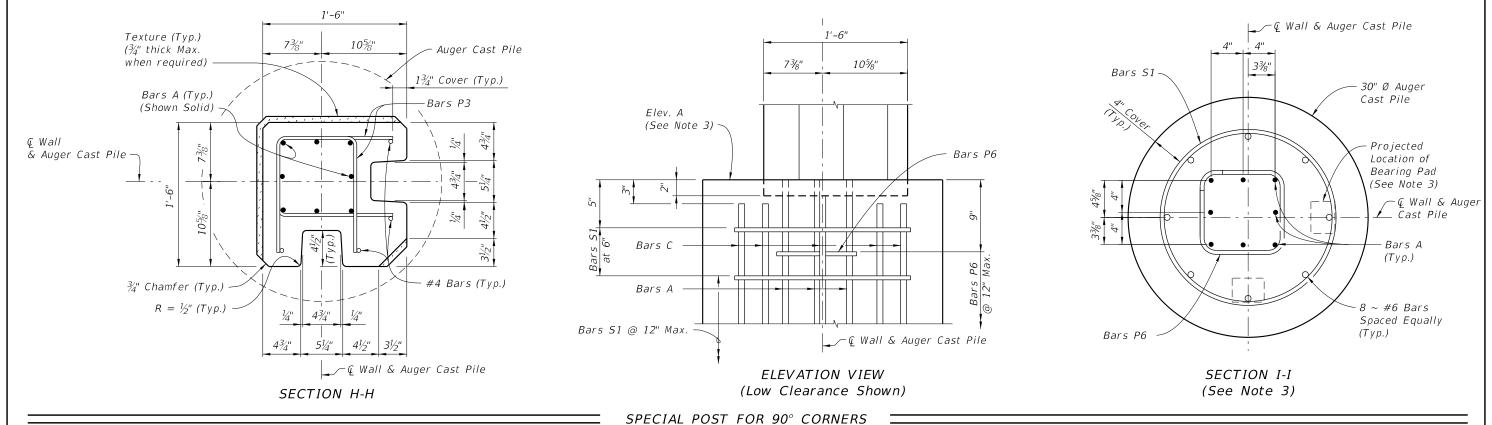
SECTION J-J

PRECAST OPTION - STANDARD POST DETAILS

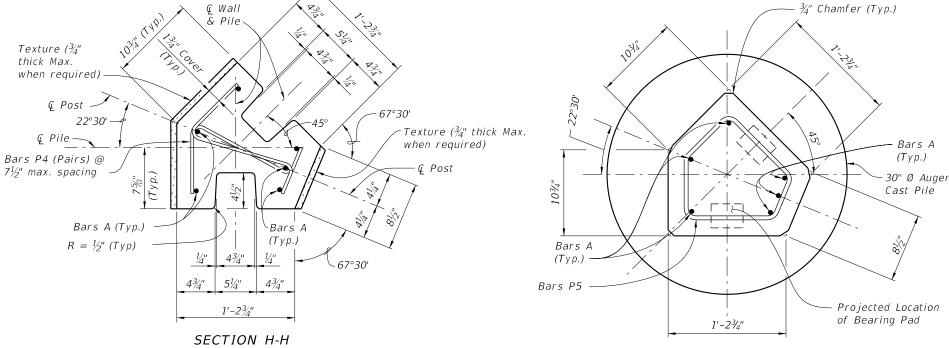
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SECTION I-I (Precast Collar)

BAR BENDING DETAILS 10½ 10½" BAR P3 BAR P6 (90° Corner) (90° Corner) Bar Length = $3' - 3^{1}/3''$ Bar Length = 4'-2'' $1'-2\frac{1}{2}'$ BAR P4 BAR P5 (45° Corner) (45° Corner) Bar Length = $2'-3\frac{1}{2}''$ Bar Length = $4'-6^{1/2}$ "

All bar dimensions in bending diagrams are out-to-out. All bars not shown in the bending diagrams are straight.

SPECIAL POSTS FOR 45° CORNERS

PRECAST OPTION - SPECIAL CORNER POSTS

LAST REVISION 11/01/17

1. For Reinforcing Steel Sizes, and Foundation Dimensions, see Table 1 Sheet 6.

3. The Bearing area beneath Neoprene Pads is formed by top of Auger Cast Pile Grout.

2. For location of Section H-H and I-I, see Sheet 6.

DESCRIPTION:

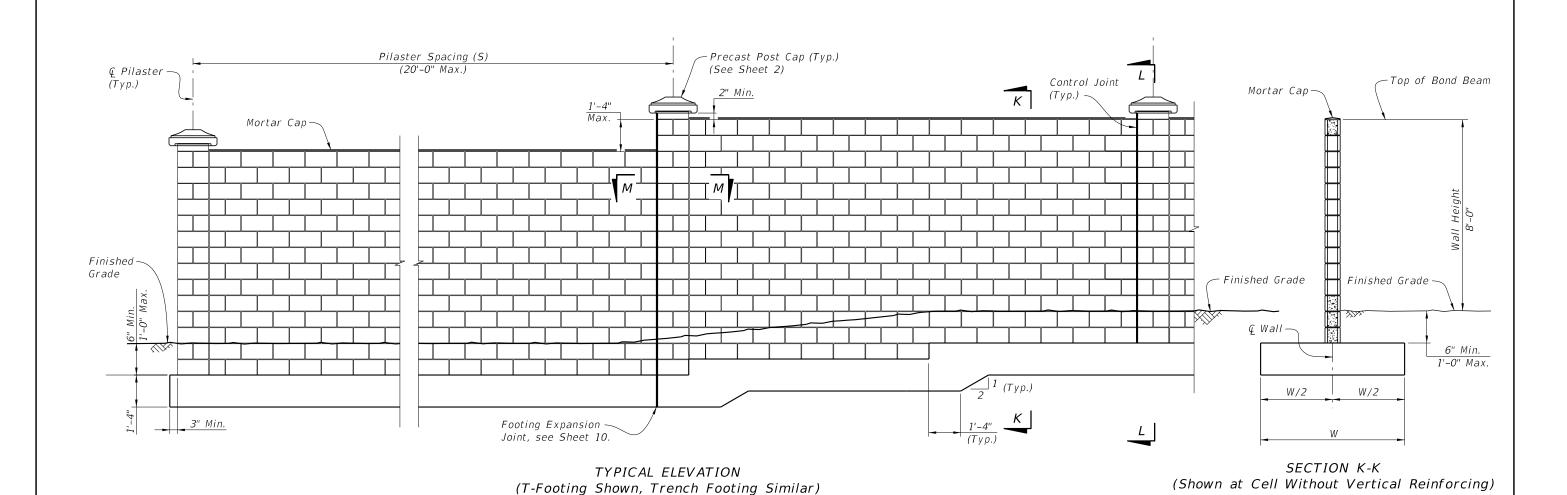
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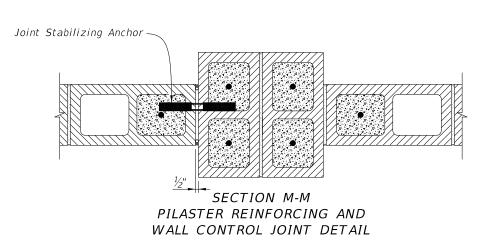
SHEET 8 of 10



		Tab	le 2								
Wind Speed Category	Masonry	walls	Foundations								
	(8x8)	x16)	Bars	T-Footing Width	Trench Footing						
	Bars V1	SV Spacing	F1 & F2	(W)	Depth (D)						
130	#5	2'-8"	#5	4'-4"	5'-6"						
150	#5	2'-0"	#5	5'-0"	6'-4"						
170	#5	1'-4"	#5	6'-0"	7'-0"						

- 1. End vertical reinforcing bars $1\frac{1}{2}$ " from top of bond beam blocks and horizontal bars $1\frac{1}{2}$ " from edge of control joints.
- 2. Do not continue horizontal #4 Bond beam reinforcing through control joint.
- 3. Use stainless steel joint stabilizing anchors spaced at 16" vertically at all control joints. Install per manufacturers instructions.
- 4. Seal Control Joints with backer rod and Type "A" silicone sealant (top and both sides).
- 5. See Sheet 10 for Bar placement details.
- 6. For Pilaster Cap Details, see Sheet 2.

DESCRIPTION:



Length = 5'-2''All bar dimensions in bending diagram are out to out. All bars not shown in the bending diagrams are straight.

MASONRY OPTION

2'-0"

BAR F1

BAR BENDING DETAIL

REVISION 11/01/17

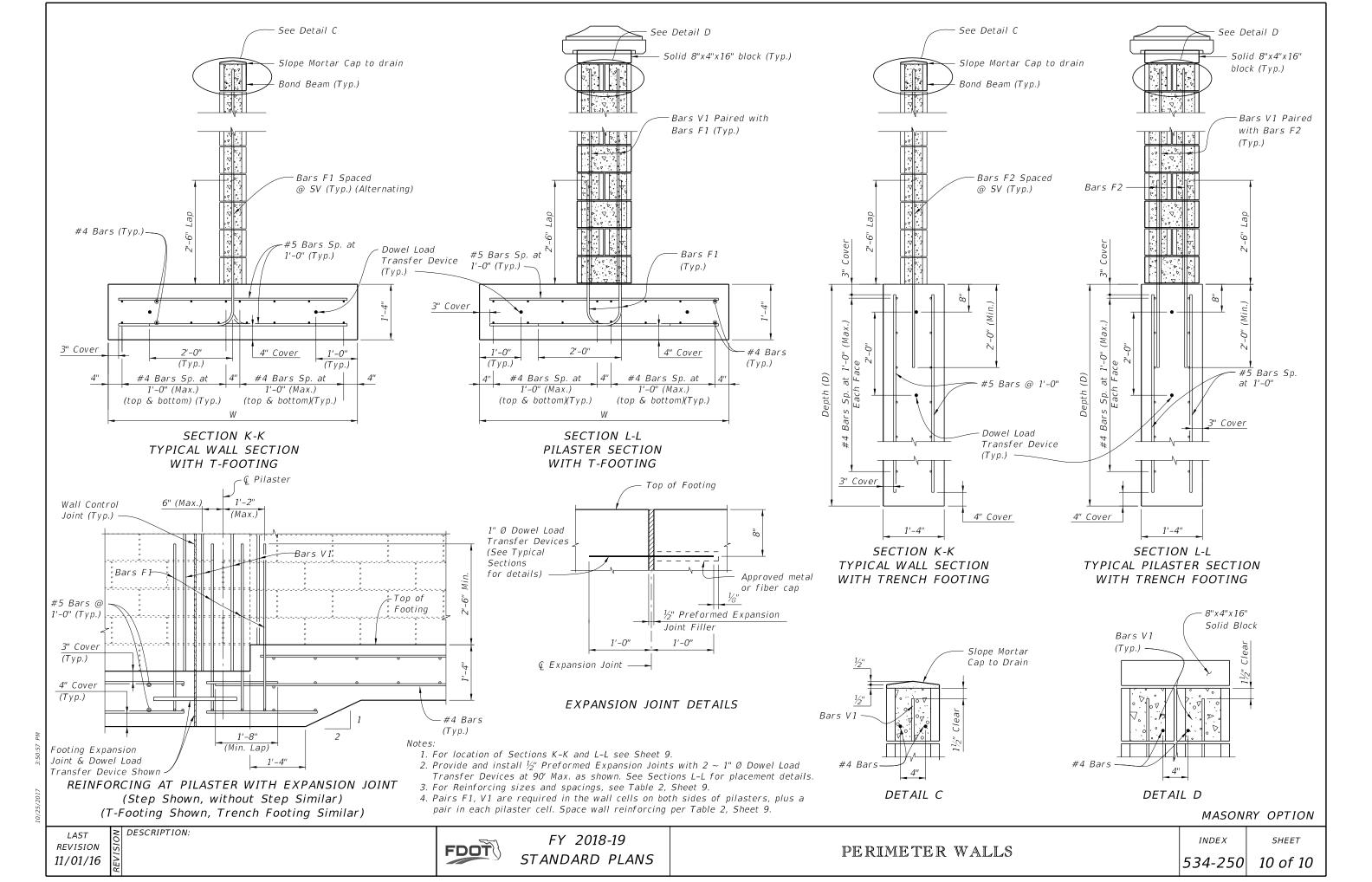
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PERIMETER WALLS



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Barrier Delineators - Post Mounted; 22 Clear Space - Reduced Post Spacing for Hazards;	21	Modified Mount – Encased Post for Shallow Mount;
22 Clear Space - Reduced Post Spacing for Hazards;		Modified Mount - Frangible Leave-Out for Concrete Surface Mount
		Barrier Delineators - Post Mounted;
5%" Button-Head Bolt System	22	Clear Space - Reduced Post Spacing for Hazards;
		⅓" Button–Head Bolt System

CHEET NO CONTENTS

LAST REVISION 11/01/17

DESCRIPTION:

F

FDOT

FY 2018-19 STANDARD PLANS

GENERAL NOTES:

1. INSTALLATION: Construct guardrail in accordance with Specification Section 536.

This Index, along with the plans and the manufacturers' drawings on the Approved Products List (APL), is sufficiently detailed for installation of General Guardrail, Low-Speed Guardrail, End Treatment assemblies, and their connecting options shown herein. This precludes requirements for shop drawing submittals unless otherwise specified in the plans.

- 2. COMPATIBILITY: The General Guardrail in this Index is based on the Midwest Guardrail System (MGS) design, with an approximate height of 31" at the top of the Panel (2'-1" mounting height at vertical © of Panel) and a midspan panel splice as shown on Sheet 2. Guardrail components included on the APL, which are compatible with this Index, may also be identified as 31" or MGS Guardrail.
- 3. STANDARD COMPONENTS: Standard guardrail components, including posts, panels, and bolt systems, are based upon English unit conversions of the AASHTO-AGC-ARTBA Joint Committee Task Force 13 Report: A Guide to Standardized Highway Barrier Hardware (http://www.aashtotf13.org/Barrier-Hardware.php).
- 4. BUTTON-HEAD BOLTS: Install Button-Head Bolts where indicated using bolts, nuts, and washers as defined on Sheet 22. Place washers under nuts; washers are optional against steel flanges. Do not place washers between bolt heads and panels, except where otherwise shown in this Index.
- 5. HEX-HEAD BOLTS: Install Hex-Head Bolts where indicated using bolts, nuts, and washers in accordance with material properties of Specification Section 967. Place washers under nuts; washers are optional against steel flanges.
- 6. MISCELLANEOUS ASPHALT PAVEMENT: Install Miscellaneous Asphalt Pavement where indicated with a tolerance of $\pm \frac{1}{2}$ " depth and in accordance with Specification Section 339.
- 7. ADJACENT SIDEWALKS & SHARED USE PATHS: When guardrail posts are placed within 4'-0" of a sidewalk or shared use path, use timber posts, or use steel posts only if treated with Pipe Rail as shown on Sheet 20.

When timber posts are used, one of the following safety treatments is required for the bolt(s) protruding from the back face of the posts:

- a. After tightening the nut, trim the protruding post bolt flush with the nut and galvanize per Specification Section 562.
- b. Use post bolts 15" in length and countersink the washer and nut between 1" and 11_2 " deep into the back face of the post.
- c. Use 15" post bolts with sleeve nuts and washers.

When End Treatment posts are within 4'-0" of a sidewalk or shared use path, steel posts are not permitted within the End Treatment segment. Terminate the Pipe Rail outside of End Treatment segments, as noted per Sheet 20.

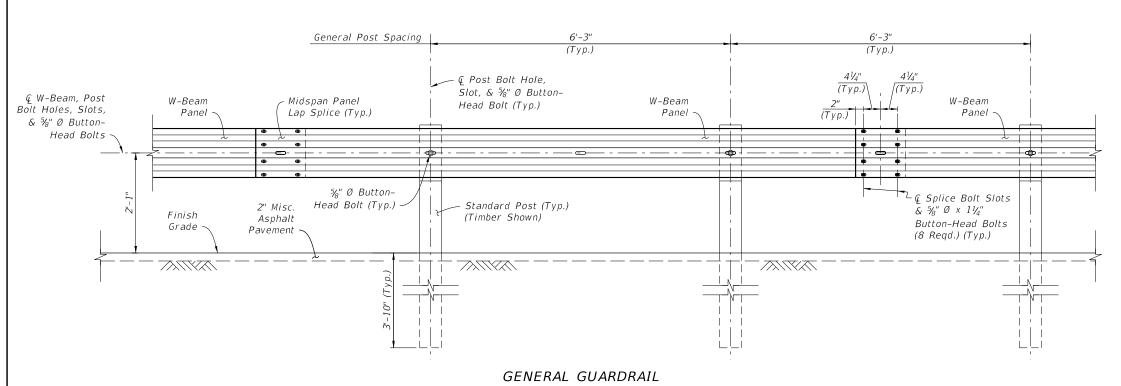
- 8. NESTED W-BEAM: Where called for in the plans, install two W-Beam Panels mounted flush per location, securing all panels with Button-Head Bolts threaded through aligned slots and holes. 2" Button-Head Bolts are permitted for panel splice locations.
- 9. CONNECTION TO RIGID BARRIER: The connections to Rigid Barrier in this Index only apply to newly constructed bridge Traffic Railing and Concrete Barrier or where the complete Approach Transition Connection to Rigid Barrier shown herein can be installed without conflicting with existing Traffic Railings, structures, or approach slabs.

For connecting guardrail to existing bridge Traffic Railings, see the layouts and details of Indexes 536-002, 521-404, and 421-405.

- 10. CONNECTION TO EXISTING GUARDRAIL: Where a transition to existing guardrail at 27" height is required, linearly transition the guardrail height over a distance ranging from 25'-0" to 31"-3". Provide an immediate transition to the required midspan splice using the available panel options on Sheet 4 (9'-4\%" or 15'-7\%" panel).
- 11. PLANS CALLOUTS: Begin/End Station labels are shown throughout this Index as they correspond to the station and offset callouts specified in the plans.

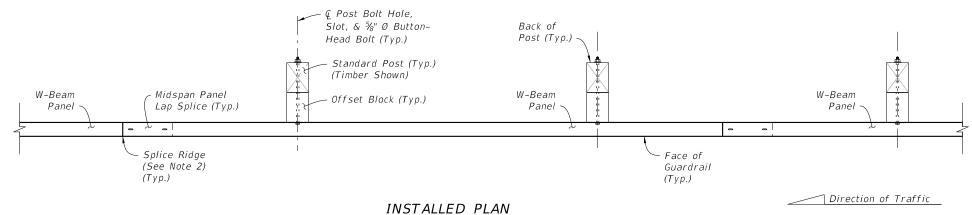
In the plans, Begin/End Guardrail Station refers to the General TL-3 Guardrail Pay Item, and it may be abbreviated as Begin/End GR. Station. Where the Low-Speed TL-2 Guardrail Pay Item is specifically required, the callout in the plans will then specify Begin/End TL-2 GR. Station.

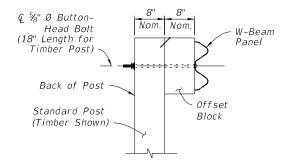
12. QUANTITY MEASUREMENT: Measure guardrail and corresponding components as defined in Specification Section 536. The Guardrail length is measured along the centerline of installed Panels, between the points labeled Begin/End Guardrail Station shown on the following Index Sheets and defined in the plans (typically measured from the © of the panel's post bolt slots at the approach/trailing ends).





INSTALLED ELEVATION





INSTALLED SECTION

NOTES:

1. GENERAL: Install the General Guardrail configuration where indicated in the plans. This may include tapered segments if called for in the plans.

Use 12'-6" or longer W-Beam Panels. A single 6'-3" Panel may be used at the end of the run to meet the nominal Begin/End Guardrail Sta. requirements.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 21 for additional post mounting options.
- 8. DEFINED SEGMENTS: The General Guardrail shown provides the base configuration, including Post Spacing and splice locations, for Defined Segment modifications where indicated in the plans and using the Guardrail Types, Sections, and/or hardware as shown in this Index (e.g. Double Faced W-Beam, Modified Thrie-Beam, Deep Posts at Slope Breaks, Pipe Rail, Rub Rail, or Reduced Post Spacing for Hazards).

GENERAL, TL-3 GUARDRAIL DETAILS

REVISION 11/01/17

DESCRIPTION:

FDOT

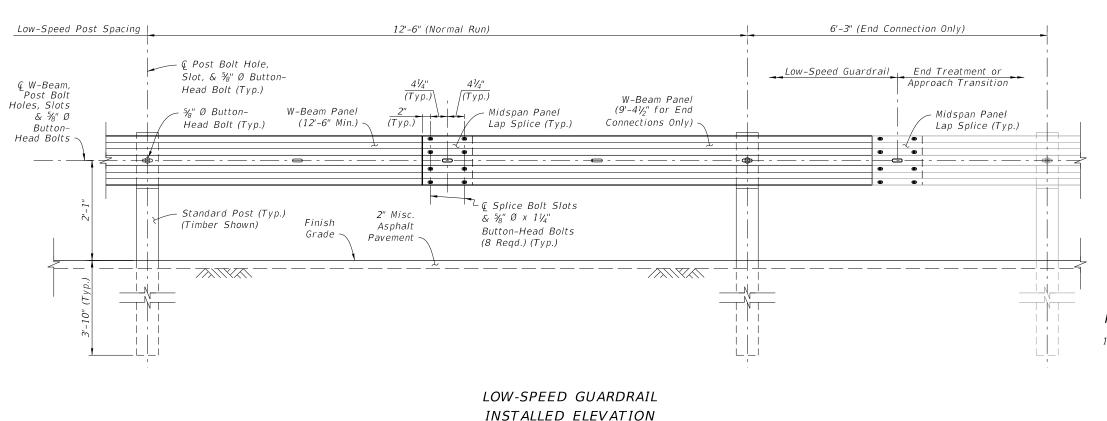
FY 2018-19 STANDARD PLANS

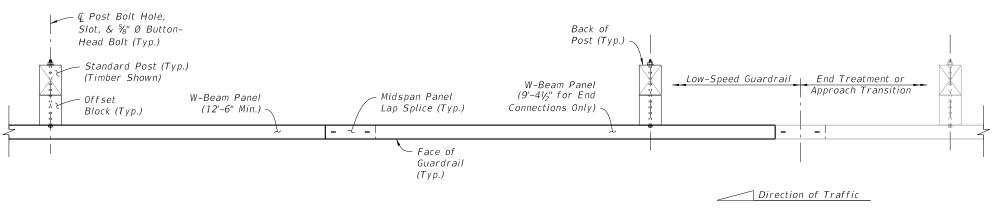
INDEX

SHEET

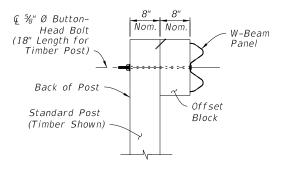
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GUARDRAIL









INSTALLED SECTION

NOTES:

1. GENERAL: Install the Low-Speed Guardrail configuration where indicated in the plans. Low-Speed Guardrail may include tapered segments if called for in the plans.

Use 12'-6" or 25'-0" W-Beam Panels for normal spans, and use 9'-4½" Panels for end connections to adjoining segments as shown. A single 6'-3" Panel may be used at the end of the Low-Speed Guardrail run along with a single reduced 6'-3" post spacing to meet the nominal Begin/End Guardrail Sta. required.

Where a differing guardrail configuration is required for constructability beyond the options shown in this Index or the Plans, obtain approval from the Engineer prior to installation.

2. MIDSPAN PANEL LAP SPLICE: For proper structural function, place all Lap Splices at midspan unless otherwise indicated.

Lap the Panels with the Splice Ridge oriented downstream of the final Direction of Traffic in the nearest traffic lane. For reverse lane conditions, orient the Splice Ridge downstream of the lane direction with the highest traffic volume. Orienting Lap Splices for Temporary Traffic Control phasing is not required.

- 3. CONNECTION DETAILS: Connections to End Treatments, Approach Transitions, or other segment types are defined in the following Index Sheets, APL Drawings, or the plans.
- 4. W-BEAM PANEL DETAILS: See Sheet 4.
- 5. POST & OFFSET BLOCK DETAILS: See Sheet 5.
- 6. GUARDRAIL SECTIONS: For Sections showing typical mounting heights, grading, and lateral offsets in relation to adjacent roadway features, see Sheet 6.
- 7. MODIFIED MOUNTS: Where concrete structures, concrete sidewalk, or shallow depth conditions are encountered, see Sheet 21 for additional post mounting options.
- 8. RESTRICTIONS: Low-Speed Guardrail segments are not permitted for use with items including, but not limited to, Double Faced W-Beam, Modified Thrie-Beam, Deep Posts at Slope Breaks, Pipe Rail. and/or Rub Rail.

LOW-SPEED, TL-2 GUARDRAIL DETAILS

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

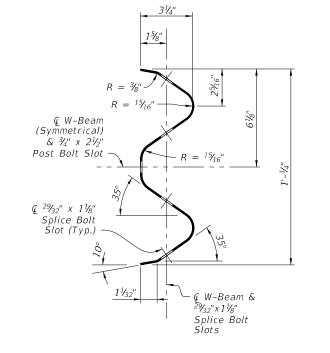
FY 2018-19 STANDARD PLANS

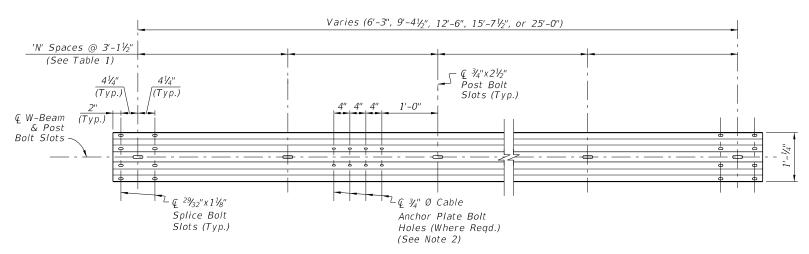
GUARDRAIL

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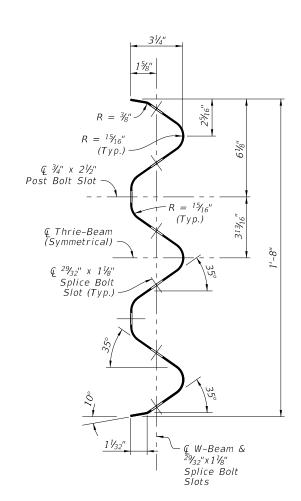


W-BEAM PANEL ELEVATION

PANEL SUMMARY TABLE:

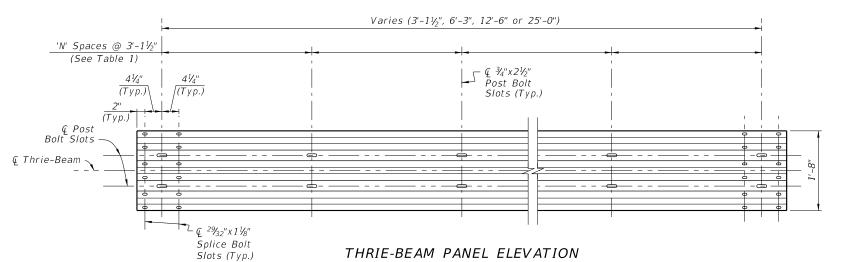
Panel Type	Number of Spaces 'N'	Gauge
6'-3" W-Beam	2	12
9'-4½" W-Beam	3	12
12'-6" W-Beam	4	12
15'-7½" W-Beam	5	12
25'-0" W-Beam	8	12
3'-1½" Thrie-Beam	1	10
6'-3" Thrie-Beam	2	12
12-6" Thrie-Beam	4	12
25-0" Thrie-Beam	8	12
Thrie-Beam Trans.	2	10

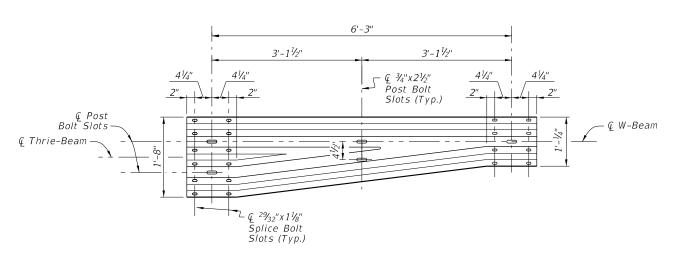
W-BEAM PANEL SECTION



THRIE-BEAM PANEL SECTION

DESCRIPTION:





THRIE-BEAM TRANSITION PANEL ELEVATION (Reverse Direction Similar by Opposite Hand)

NOTES:

1. MATERIALS:

Use corrugated steel panels in accordance with Specification Section 967 and made from either Class A, 12 gauge steel or Class B, 10 gauge steel as specified in the 'Panel Summary Table'

2. CABLE ANCHOR PLATE BOLT HOLES: Include ¾" Ø Cable Anchor Plate Bolt Holes only where required for installation of the Cable Anchor Plate shown on Sheet 9, 10, & 11.

 29 /₃₂" x 11/8" slots may substitute for the 3 /4" Ø holes shown.

> W-BEAM AND THRIE-BEAM PANEL DETAILS

LAST **REVISION** 11/01/17

FDOT

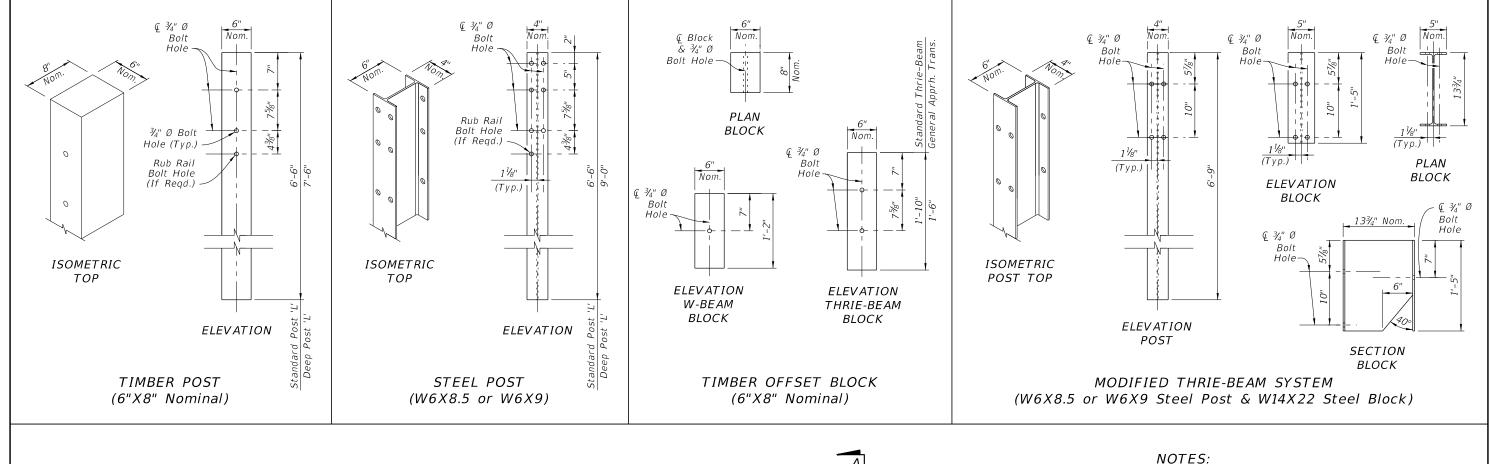
FY 2018-19 STANDARD PLANS

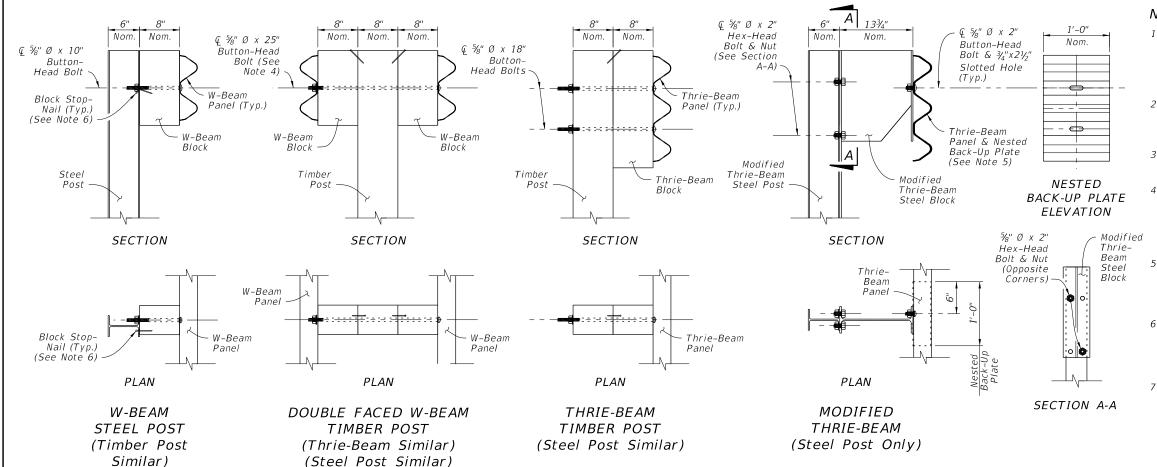
GUARDRAIL

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- 1. STANDARD POSTS: Where Standard Posts are called for in this Index, use either a Timber Post or Steel Post at the Length, 'L', shown for Standard Posts. Use a single post material type consistently per each run of guardrail. Only where specified in the Plans, use the Deep Post 'L' for Slope Break Conditions as shown on Sheet 6.
- 2. OFFSET BLOCKS: For each Panel type, install the corresponding Offset Block type as shown. For General, TL-3 (Single Faced) Approach Transitions only, use the 1'-6" Thrie-Beam Block (See Sheet 13).
- 3. BOLT HOLES: 3/1 Ø Bolt Holes shown in posts within this Index may be substituted with ${}^{13}\!\!/_{16}$ " Ø Bolt Holes.
- 4. DOUBLE FACED GUARDRAIL: Orient Post Bolts with the Button-Head located on the side nearest the traffic lane. The bolt's threaded portion is not permitted to extend beyond 3/4" from the face of the tightened nut; trim the threaded portion as needed and galvanize in accordance with Specification Section 562.
- 5. MODIFIED THRIE-BEAM NESTED BACK-UP PLATE: At each post connection, install a Nested Back-up Plate between the Thrie-Beam Panel and the post. The Nested Back-up Plate has a cross-section and material matching the Thrie-Beam Panel Section.
- 6. BLOCK STOP-NAIL: Drive one nail per Standard Offset Block as shown to prevent Block rotation. Use steel 31/2" Type 16d nails with ASTM A153 hot-dip galvanization. For steel posts, drive the nail through the unused flange Bolt Hole and bend the nail so its head contacts the flange.
- 7. MATERIALS: Use timber and steel posts and offset blocks in accordance with Specification Section 967. Composite offset blocks may be substituted as approved on the APL. Use a single offset block type consistently per each run of guardrail. Steel offset blocks are only permitted for Modified Thrie Beam.

POST AND OFFSET BLOCK DETAILS

REVISION 11/01/17

DESCRIPTION:

FDOT

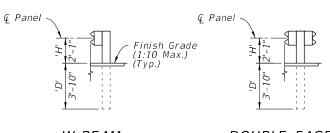
FY 2018-19 STANDARD PLANS

GUARDRAIL

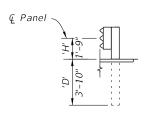
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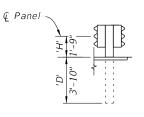
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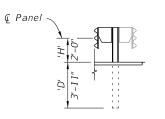
DOUBLE FACED W-BEAM W-BEAM



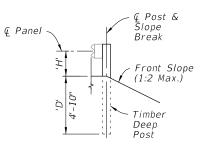
THRIE-BEAM



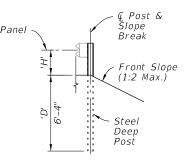
DOUBLE FACED THRIE-BEAM



MODIFIED THRIE-BEAM



SLOPE BREAK CONDITION TIMBER DEEP POST

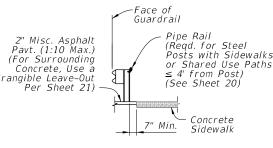


SLOPE BREAK CONDITION STEEL DEEP POST

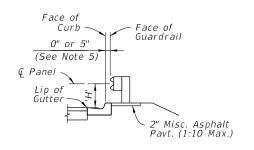
GUARDRAIL TYPES - MOUNTING HEIGHTS & POST DEPTHS:



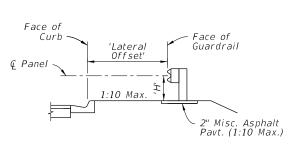
TYPICAL GRADING & PAVT. PLACEMENT DETAIL (See Note 2)



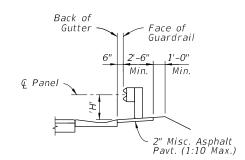
TYPICAL SIDEWALK DETAIL (Work with Other Sections as Read.)



ADJACENT TO CURB (Type F Curb Shown)



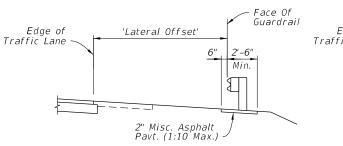
BEHIND CURB (Type F Curb Shown)



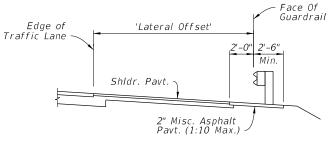
ADJACENT TO SHOULDER GUTTER

GUARDRAIL SECTIONS - TYPICAL=

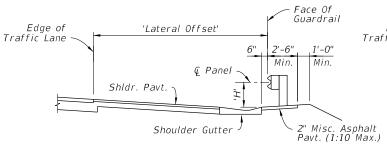
GUARDRAIL SECTIONS - CURB & GUTTER:



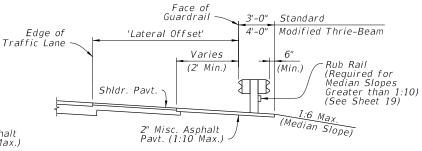
UNPAVED OR PARTIALLY PAVED SHOULDER



FULLY PAVED SHOULDER



SHOULDER GUTTER



DOUBLE FACED GUARDRAIL (Shown In Median)

GUARDRAIL SECTIONS - SHOULDERS:

GUARDRAIL HEIGHT SUMMARY TABLE:					
Type:	Min. Depth 'D':	Mounting Height 'H':	Post Length 'L':		
W-Beam (Single and Double Faced)	3'-10"	2'-1"	6'-6"		
Thrie-Beam (Single and Double Faced)	3'-10"	1'-9"	6'-6"		
Modified Thrie-Beam	3'-11"	2'-0"	6'-9"		
Timber Deep Post	4'-10"	See Above	7'-6"		
Steel Deep Post	6'-4"	See Above	9'-0"		

NOTES:

- 1. GUARDRAIL SECTIONS: Construct Sections as indicated in the plans. The details shown herein depict W-Beam Guardrail, but are applicable to the other defined Guardrail Types placed at the corresponding height, 'H'. Use components per Sheets 4 & 5. Steel and timber post types are interchangeable unless otherwise defined. The 1:10 Max. cross slope shown is the maximum slope permitted for proper quardrail function, but project-specific cross slope requirements are governed per the plans.
- 2. TYPICAL GRADING & PAVEMENT PLACEMENT DETAIL: Construct features as depicted except where superceded by specific Guardrail Sections or the plans. Place the Slope Break a Minimum of 2' behind the post. For Deep Posts, the slope break may be placed at the @ Post with the 2" Miscellaneous Asphalt Pavement omitted.
- 3. SLOPE BREAK CONDITION: Install Deep Posts only where called for in the plans. Deep Posts are only permitted where post spacing is 6'-3" or less.
- 4. LATERAL OFFSETS: The Lateral Offsets shown are governed by the station and offset call outs for Face of Guardrail, as shown in the plans.
- 5. ADJACENT TO CURB: Place the Face of Guardrail consistently offset either flush with the Face of Curb or 5" behind the Face of Curb, as indicated by the plans station and offset callout. For offset changes, transition the Face of Guardrail as shown in the plans.

GUARDRAIL SECTIONS

REVISION 11/01/17

DESCRIPTION:

FDOT

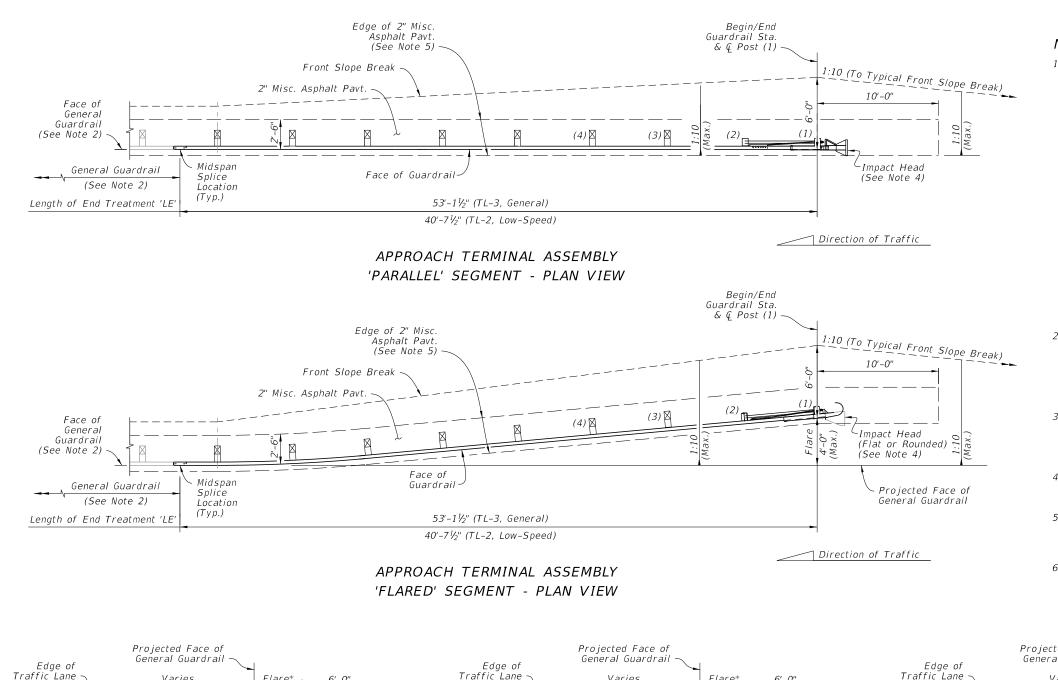
FY 2018-19 STANDARD PLANS

GUARDRAIL

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NOTES:

1. INSTALLATION: Locate Approach Terminals where called for in the plans, with the Post (1) © placed at the Begin/End Guardrail Station indicated in the plans.

The Plan Views shown herein are schematic only, showing basic geometry for Approach Terminals listed on the APL. The predefined Length of End Treatment, 'LE', includes the proprietary portion of various Approach Terminals and provides for more consistent planning of assembly installations across the differing Approach Terminal types. Forward-anchoring style Approach Terminals may vary from the planned lengths shown by up to 3'-0".

Construct Approach Terminals as shown in the APL and in accordance with the manufacturer's unique drawing details, procedures, and specifications.

Install posts in accordance with the manufacturer's drawings. The Special Posts on Sheet 21, including Special Steel Posts, Encased Posts, and Frangible Leave-Outs, are not permitted within the Approach Terminal segment unless otherwise called for in the plans.

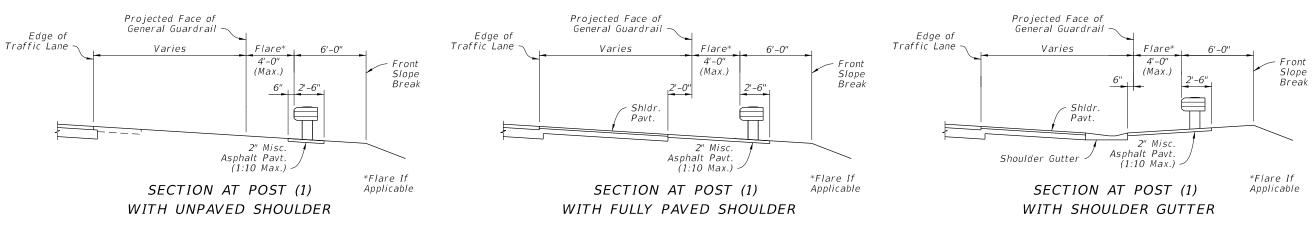
Align panel lap splices in accordance with the manufacturer's drawings, regardless of the direction of traffic.

Install adjacent grading, gutters, and/or curbing as shown herein, unless otherwise specified in the plans.

 GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments.

Approach Transitions, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.

- 3. APPROACH TERMINAL TEST LEVEL: Install either a Test Level 3 (TL-3) or Test Level 2 (TL-2) Approach Terminal as specified in the plans. TL-3 Approach Terminals may substitute for TL-2 Approach Terminals unless the substitution is specifically prohibited in the plans. TL-2 Approach Terminals may not substitute for TL-3 installations.
- 4. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification Section 536.
- 5. 2" MISCELLANEOUS ASPHALT PAVEMENT: The Plan Views shown herein depict the Unpaved Shoulder condition. For Fully Paved Shoulder and Shoulder Gutter conditions, extend the 2" Misc. Asphalt Pavement as shown in the corresponding 'Section at Post (1)' details below.
- 6. 'CURBED' AND 'DOUBLE FACED' GUARDRAIL SEGMENTS: See Sheet 8.



END TREATMENT -APPROACH TERMINAL GEOMETRY PARALLEL AND FLARED

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

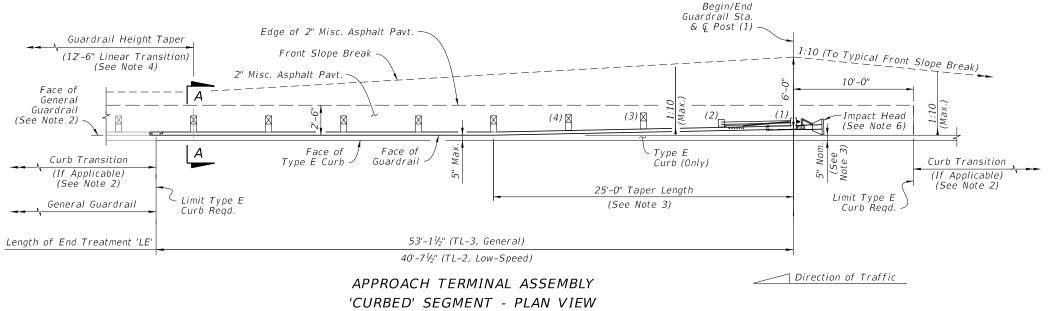
FY 2018-19 STANDARD PLANS

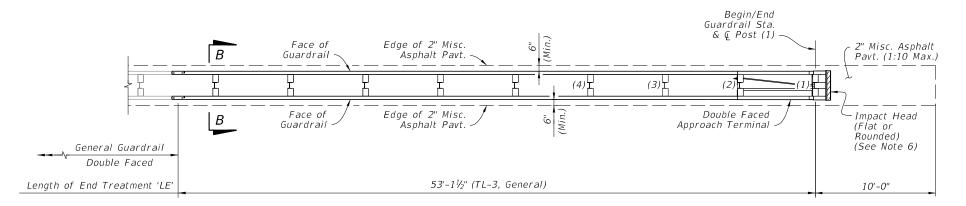
GUARDRAIL

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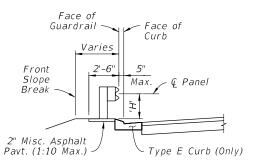
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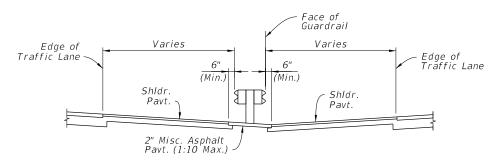




APPROACH TERMINAL ASSEMBLY 'DOUBLE FACED' SEGMENT - PLAN VIEW



'CURBED' SECTION A-A (Height, 'H', Measured from Misc. Asphalt Pavt.)



'DOUBLE FACED' SECTION B-B (1:10 Slope or Flatter Reqd.)

NOTES:

- 1. GENERAL: See Notes 1 through 3 on Sheet 7.
- 2. CURBED SEGMENTS: Type E curb is required within the limits shown. When a different curb type is called for outside of the Type E curb limits, transition the curb shape linearly, over a nominal distance ranging 5'-0" to 10'-0"
- 3. TAPER LENGTH: For Curbed Segments, taper the guardrail away from the roadway where shown to place the inside edge of the Impact Head at 5" behind the face of the curb. Where additional lateral offset is required to fit the Approach Terminal Assembly hardware, such as a soil plate, place the Impact Head as close to the curb as the hardware allows, not to exceed 2'-0" from the face of curb.
- 4. GUARDRAIL HEIGHT TAPER: For Curbed Segments, the connecting General Guardrail Mounting Height, 'H', is typically measured from the Lip of Gutter (See Sheet 6 Guardrail Sections, 'Adjacent to Curb'), while the End Terminal Assembly 'H' is measured from the Misc. Asphalt Pavt. (See Section A-A). Linearly taper the difference in Mounting Height over a minimum length of 12-6", starting where indicated herein.
- 5. DOUBLE FACED SEGMENT: Connect to Double Faced General Guardrail. Use consistent Posts and Offset Block types as specified in the APL drawings over the entire Length of End Treatment, 'LE'. Posts and Offset Blocks in the adjoining General Guardrail segment may be different from those inside of the 'LE'. A change in post type between timber and steel is permitted, immediately outside of the 'LE' segment.

Maintain the 1:10 maximum grading as shown in Section B-B throughout segment 'LE'. Where required, transition to differing adjacent slopes linearly, over a minimum longitudinal length of 25'-0".

- 6. IMPACT HEAD END DELINEATOR: Apply Yellow Retroreflective Sheeting to the nose of the End Terminal in accordance with Specification Section 536.
- 7. SINGLE FACED 'PARALLEL' AND 'FLARED' SEGMENTS: See Sheet 7.

END TREATMENT -APPROACH TERMINAL GEOMETRY CURBED AND DOUBLE FACED

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

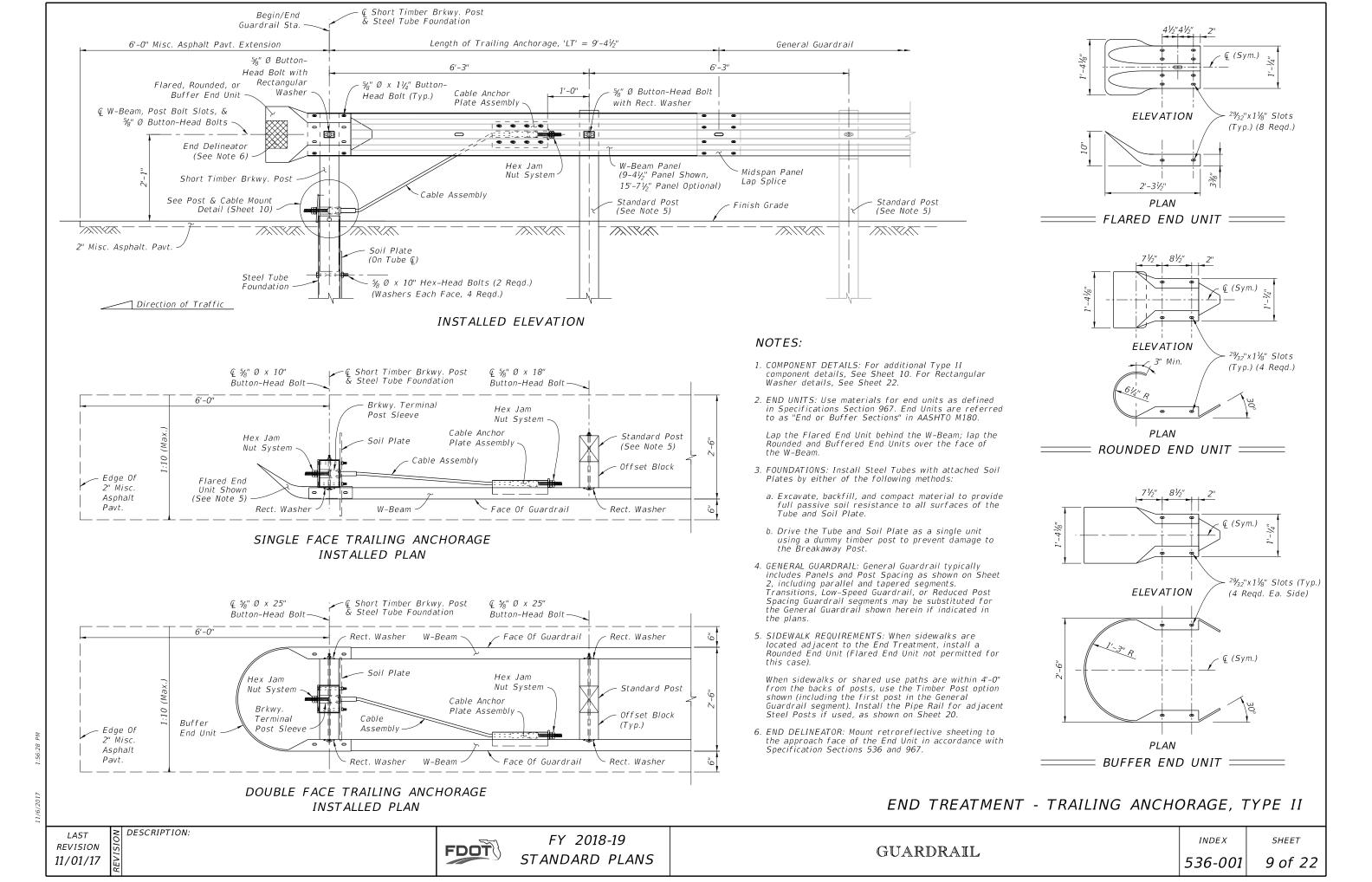
FY 2018-19 STANDARD PLANS

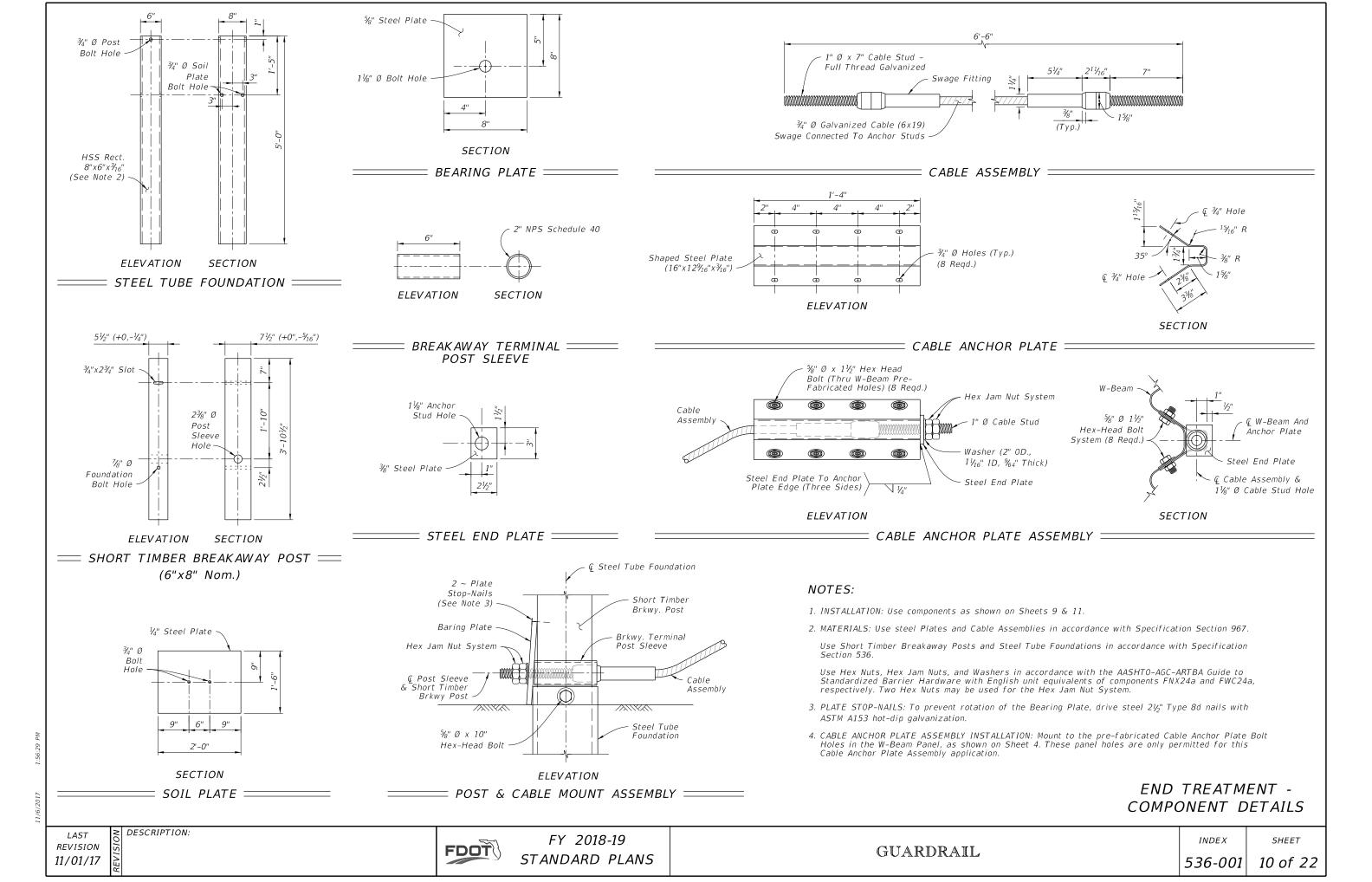
GUARDRAIL

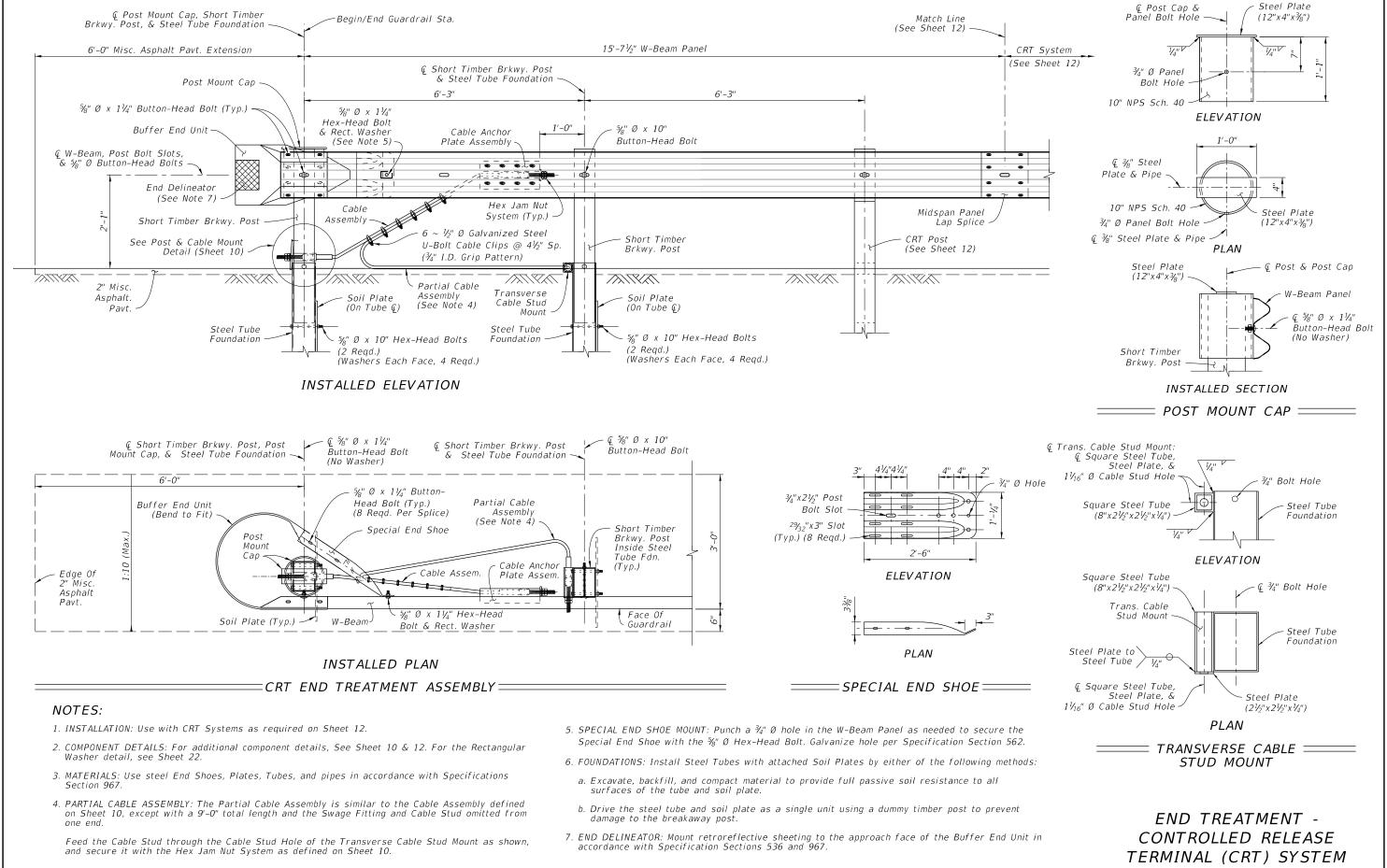
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LAST REVISION 11/01/17

DESCRIPTION:

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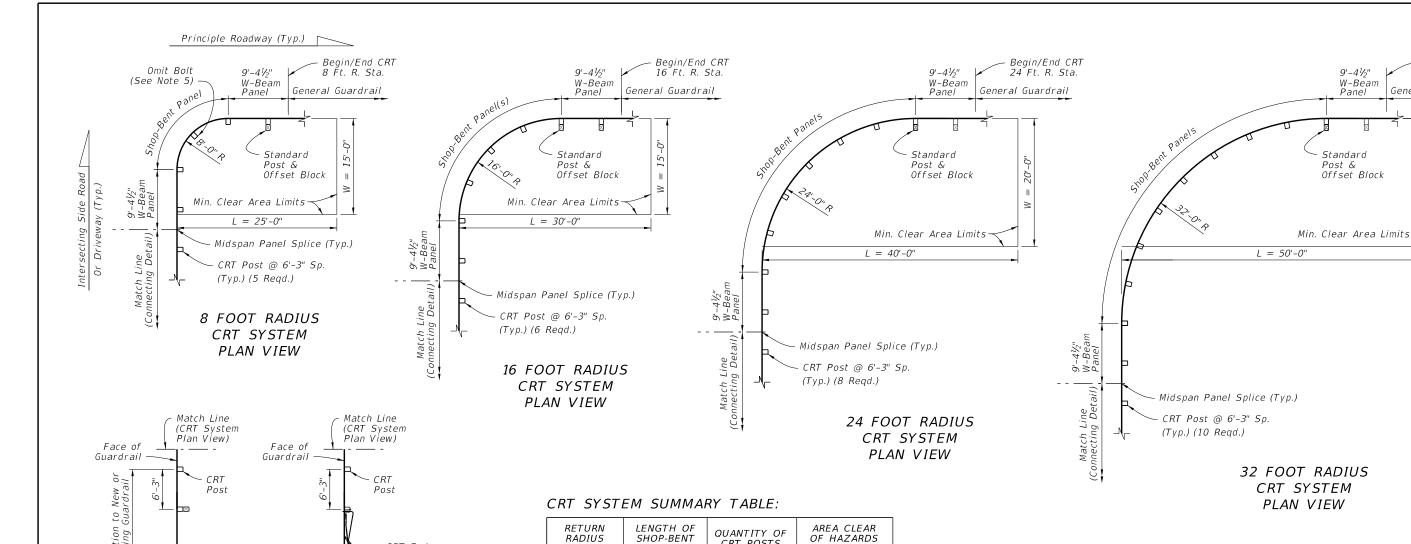
FY 2018-19 STANDARD PLANS

GUARDRAIL

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END TREATMENT OPTION

CONTINUING OPTION

(FT.) PANEL(S) (FT.) Treatment Begin/End Assembly

RADIUS

32

(FT.)	PANEL(S) (FT.)	CRI POSIS	'L' x 'W' (FT.)
8	12.5	5	25 x 15
16	25.0	6	30 x 15
24	37.5	8	40 x 20

50.0

CRT POSTS

10

OF HAZARDS

50 x 20

CONNECTING DETAIL=

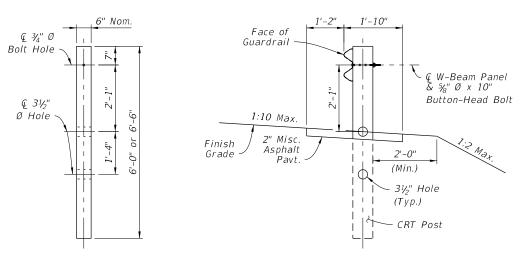
Guardrail Sta.

NOTES:

1. INSTALLATION: Construct the specified radius layout and Connecting Detail option as shown in the plans.

(See Sheet 11)

- 2. MIN. CLEAR AREA: Keep the area behind the CRT free of fixed objects and aboveground hazards within the Min. Clear Area limits shown. Maintain a slope not steeper than 1:10 for a minimum 2' behind the posts, and maintain a slope not steeper than 1:2 beyond 2'
- 3. APPROACH GRADING: Maintain grading on the roadway side of the quardrail face at a maximum slope of 1:10.
- 4. MATERIALS: For CRT Posts, use Timber Post material in accordance with Specification Section 967. Use steel panels and hardware in accordance with Specification Section 967.
- 5. BOLT OMISSION: For the 8 Foot Radius CRT System only, do not place a panel-to-post mount bolt at the center CRT Post (omit the \(\frac{\pi}{8} \)" Button-Head Bolt only at the location shown).
- 6. SHOP-BENT PANELS: Install Shop-Bent panel(s) where indicated using 12'-0" or 25'-0" W-Beam Panels. Splice at post locations within the CRT radius using the General configuration of $\frac{9}{8}"$ Ø Button-Head Bolts (8 reqd. per splice).
- 7. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments. Approach Transitions, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.



CRT POST ELEVATION (6"x8" Nom. Timber)

CRT INSTALLED SECTION

LAYOUT FOR CONTROLLED RELEASE TERMINAL (CRT) SYSTEMS -SIDE ROADS AND DRIVEWAYS

REVISION 11/01/17

FDOT

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GUARDRAIL

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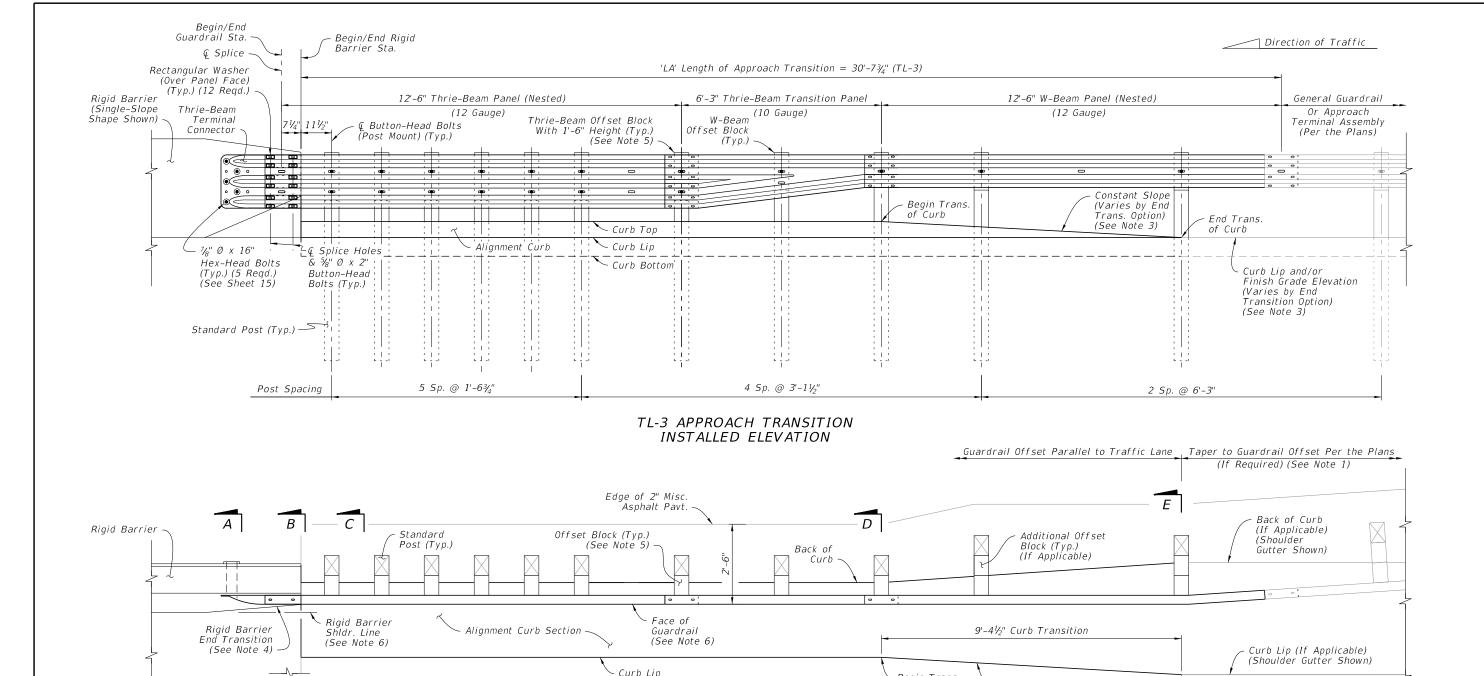
DESCRIPTION:

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Begin/End CRT

32 Ft. R. Sta.

General Guardrail



NOTES:

1. INSTALLATION: Construct the Approach Transition segment where indicated in the plans. The required offset of the connecting adjacent guardrail is shown in the

The Layouts given on Sheet 17 provide basic schemes for connections to adjacent guardrail, where a taper to a differing guardrail offset may be required. If the adjacent guardrail segment has the same offset as the Approach Transition segment, then no taper is required.

For existing bridge connection options, see Indexes 536-002, 521-404, and 521-405.

2. SECTION VIEWS & DETAILS: For cross sections and details including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 15.

3. END TRANSITION OF CURB OPTION: The Plan and Elevation views depict an example Curb Transition to Shoulder Gutter from Section D-D to E-E, but this transition may require a different shape depending on the End Transition option indicated in the plans (Either a 'Shoulder Gutter Option', 'Raised Curb Option', or 'Flat No Curb Option'). See Sheet 15 for curb shape details.

TL-3 APPROACH TRANSITION

INSTALLED PLAN

- 4. RIGID BARRIER END TRANSITION: Taper the Rigid Barrier toe as shown. See Concrete Barrier, Index 521-001, and Traffic Railing, Indexes 521-422 and 521-428,
- 5. OFFSET BLOCKS: For Thrie-Beam post locations within the Length of Approach Transition segment, use the Timber Offset Blocks with 1'-6" height shown on Sheet 5.

For the midspan of the Thrie-Beam Transition Panel and for all other W-Beam locations shown herein, use the W-Beam Offset Blocks with 1'-2" height.

6. OFFSET: The required offset difference between the Face of Guardrail and Rigid Barrier Shoulder Line is considered negligible and may not be shown in the guardrail offset callouts in the plans. A consistent guardrail offset deviation of up to 4 inches outside of the Rigid Barrier Shoulder Line is permitted over the length 'LA'.

End Trans.

of Curb

7. GENERAL GUARDRAIL: General Guardrail typically includes Panels and Post Spacing as shown on Sheet 2, including parallel and tapered segments. Approach Terminals, Low-Speed Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the General Guardrail shown herein if indicated in the plans.

> APPROACH TRANSITION CONNECTION TO RIGID BARRIER - GENERAL, TL-3

↑ Direction of Traffic

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2018-19 STANDARD PLANS

GUARDRAIL

Constant Taper

(See Note 3)

(Varies by End Trans. Option)

(Shoulder Gutter Shown)

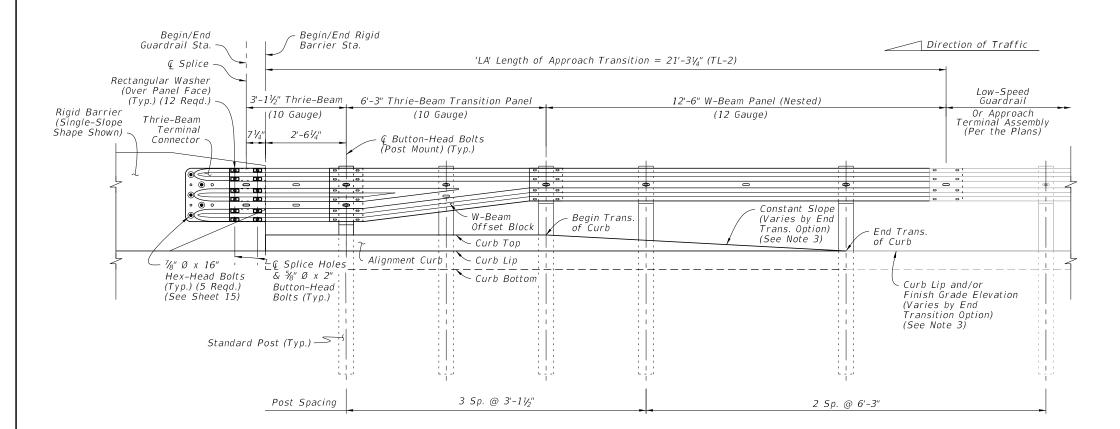
Begin Trans.

of Curb

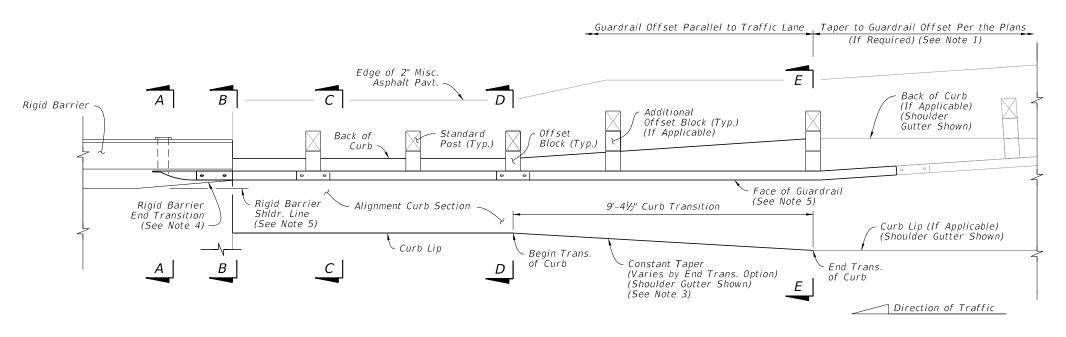
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SHEET



TL-2 APPROACH TRANSITION INSTALLED ELEVATION



TL-2 APPROACH TRANSITION INSTALLED PLAN

NOTES:

1. INSTALLATION: Construct the Approach Transition segment where indicated in the plans. The required offset of the connecting adjacent guardrail is shown in the plans.

The Layouts given on Sheet 17 provide basic schemes for connections to adjacent guardrail, where a taper to a differing guardrail offset may be required. If the adjacent guardrail segment has the same offset as the Approach Transition segment, then no taper is required.

For existing bridge connection options, see Indexes 536-002, 521-404, and 521-405.

- 2. SECTION VIEWS & DETAILS: For cross sections and details including the barrier mounting hardware, curb transition, adjacent grading, and installation dimensions, see Sheet 15.
- 3. END TRANSITION OF CURB OPTION: The Plan and Elevation views depict an example Curb Transition to Shoulder Gutter from Section D-D to E-E, but this transition may require a different shape depending on the End Transition option indicated in the plans (Either a 'Shoulder Gutter Option', 'Raised Curb Option', or 'Flat No Curb Option'). See Sheet 15 for curb shape details.
- 4. RIGID BARRIER END TRANSITION: Taper the Rigid Barrier toe as shown. See Concrete Barrier, Index 521-001, and Traffic Railing, Indexes 521-422 thru 521-428, for
- 5. OFFSET: The required offset difference between the Face of Guardrail and Rigid Barrier Shoulder Line is considered negligible and may not be shown in the guardrail offset callouts in the plans. A consistent guardrail offset deviation of up to 4 inches outside of the Rigid Barrier Shoulder Line is permitted over the
- 6. LOW-SPEED GUARDRAIL: Low-Speed Guardrail typically includes Panels and Post Spacing as shown on Sheet 3, including parallel and tapered segments. Approach Terminals, General Guardrail, or Reduced Post Spacing Guardrail segments may be substituted for the Low-Speed Guardrail shown herein if indicated in the plans.

APPROACH TRANSITION CONNECTION TO RIGID BARRIER - LOW-SPEED, TL-2

REVISION 11/01/17

DESCRIPTION:

FDOT

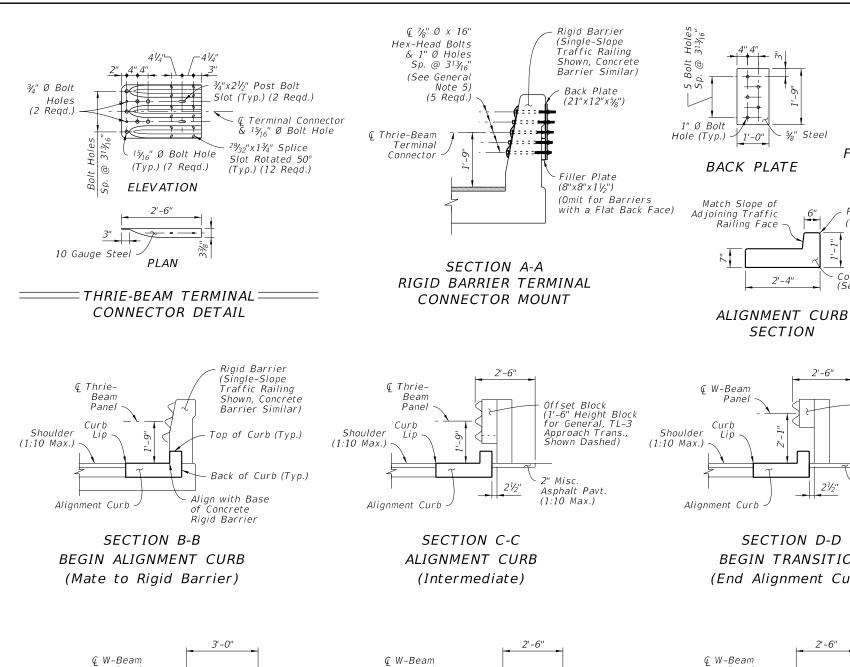
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GUARDRAIL

INDEX

SHEET

536-001|



Additional Off set

2" Misc. Asphalt Pavt.

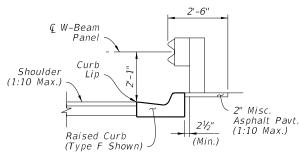
(1:10 Max.)

-Shldr

Line

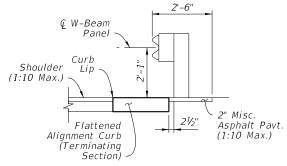
SECTION D-D BEGIN TRANSITION (End Alignment Curb)

21/2"

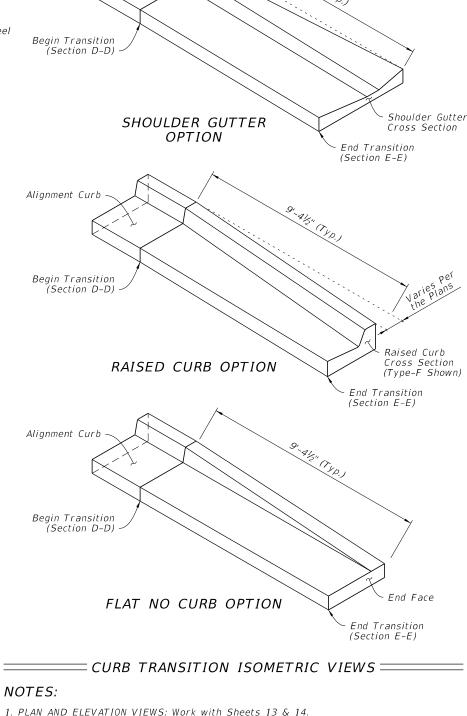


SECTION E-E **END TRANSITION** RAISED CURB OPTION

CURB TYPICAL SECTIONS



SECTION E-E **END TRANSITION** FLAT NO CURB OPTION



Alignment Curb

FILLER PLATE

 $R = \frac{3}{4}'' (Typ.)$

(Top Corners)

Concrete

(See Note 4)

Block (Typ.)

2" Misc.

Asphalt Pavt.

(1:10 Max.)

- 2. END TRANSITION OF CURB OPTION: Install one of the three End Transition types shown per Section E-E as indicated by the plans.
- 3. GRADING BEHIND POSTS: Place Slope Break a Min. 2'-0" behind the post, per Sheet 6.
- 4. MATERIALS & CONSTRUCTION: Construct the concrete Aligning Curb and Curb transition in accordance with Specification Section 520. Use steel Plates and Thrie-Beam Terminal Connectors in accordance with Specifications Section 967.

APPROACH TRANSITION CONNECTION - DETAILS

REVISION 11/01/17

FDOT

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SHEET

Curb

Shoulder Gutter

DESCRIPTION:

SECTION E-E

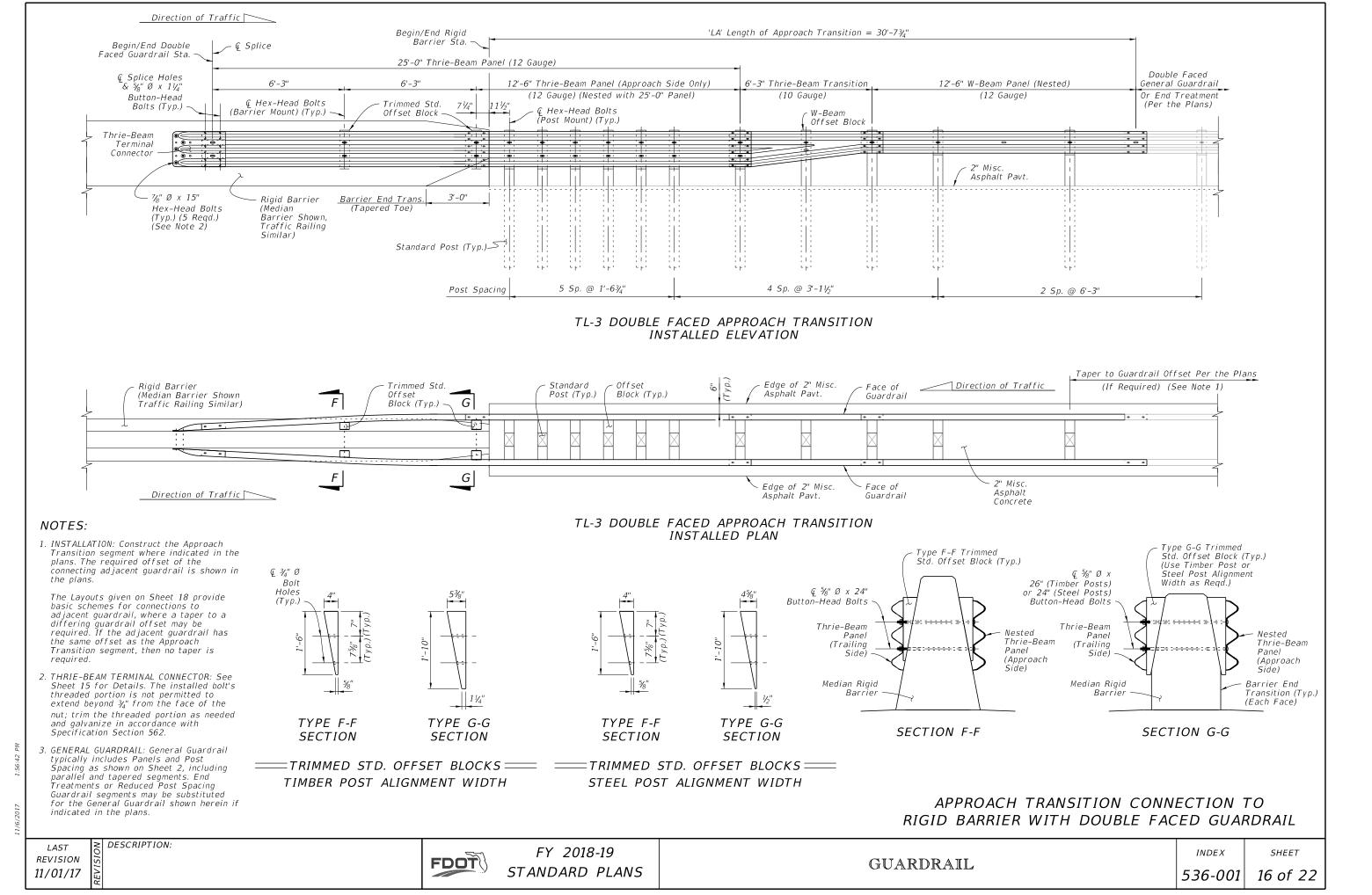
END TRANSITION

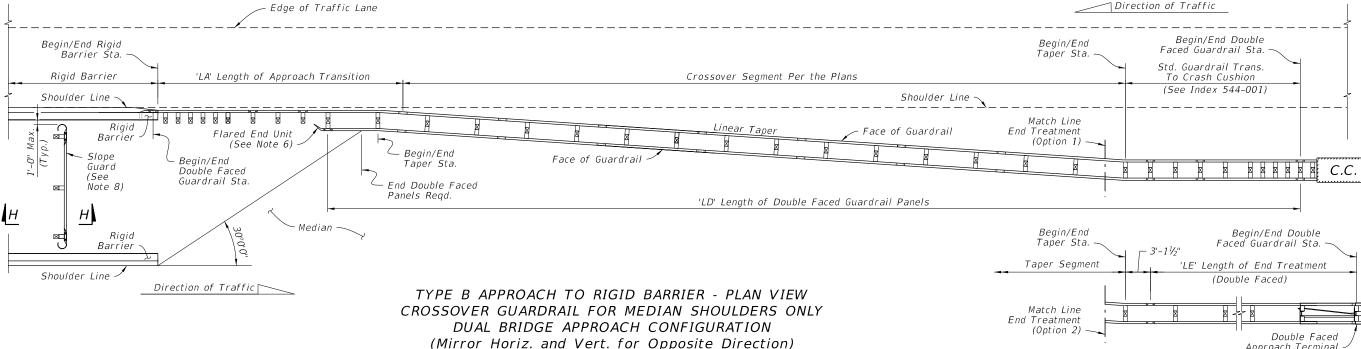
SHOULDER GUTTER OPTION

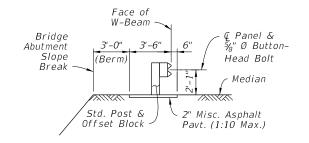
Shoulder (1:10 Max.)

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SECTION H-H BRIDGE ABUTMENT SLOPE GUARD (Between Bridges)

DESCRIPTION:

NOTES:

- 1. INSTALLATION: The Plan Views shown are schematic only, showing example geometry for connecting quardrail segments including taper locations and Double Faced Guardrail requirements as applicable. Work this Sheet with the plans, where stationing and offsets for Begin/End Guardrail, Begin/End Rigid Barrier, and Begin/End Taper are specified. For existing bridge layouts, see Index 536-002, 521-404,
- 2. GENERAL (OR LOW-SPEED) GUARDRAIL SEGMENT: Construct this segment if shown in the plans. For the case where this segment's offset differs from the Approach Transition offset, linearly taper the guardrail between the Begin/End Taper Stations and offsets as specified in the plans

For the shortest length case of a direct connection between the End Treatment and the Approach Transition, this segment may be omitted as shown in the plans.

- 3. LENGTH OF APPROACH TRANSITION 'LA': Install the Approach Transition as shown per Sheet 13 or 14 as called for in the plans.
- 4. LENGTH OF END TREATMENT 'LE': Install the Approach Terminal End Treatment as shown per Sheet 7 or 8, where called for in the plans. Use the corresponding APL drawings for construction details.
- 5. CROSSOVER GUARDRAIL (FOR TYPE B APPROACH): Install the Crossover Segment tapering linearly from the Begin Taper Sta. and offset to the End Taper Sta. and offset as specified in the plans.

6. LENGTH OF DOUBLE FACED GUARDRAIL PANELS, 'LD' (FOR TYPE B APPROACH): Terminate the Double Faced Guardrail panels as shown (based upon the 30° line measured from the hazard on the opposite side of the median). Extend the panel segment longer than the dimension 'LD' as needed for the Panel's end Bolt Slot to align with a post Bolt hole.

Install a Flared End Unit where shown, as defined on Sheet 9.

- 7. END TREATMENT OPTIONS (FOR TYPE B & C APPROACH): For Double Faced applications, use either a Double Faced Approach Terminal Assembly per Sheet 8 or a Crash Cushion per Index 544-001. For either Option, meet the 1:10 adjacent grading requirements for Approach Terminals as shown on Sheet
- 8. SLOPE GUARD: Where indicated in the plans, install a Guardrail segment between bridge approaches and offset from the bridge abutment's Slope Break as shown. Install posts at the end bolt slots of the panel system. Use post spacing of either 3'-11/3" or 6'-3", as needed to correctly fit system between barriers. The system may also be lengthened to fit by installing two Rounded End Units as defined on Sheet 9.

LAYOUT TO RIGID BARRIER -APPROACH ENDS

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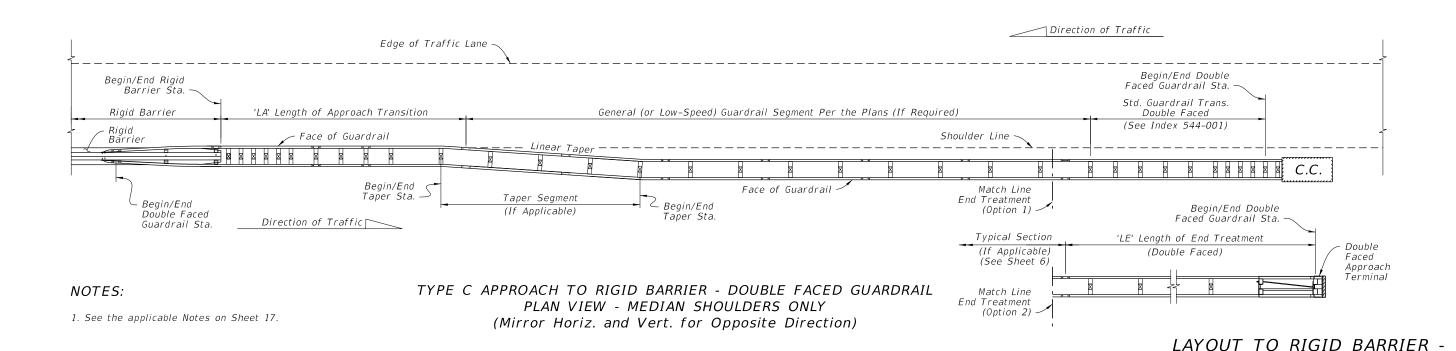
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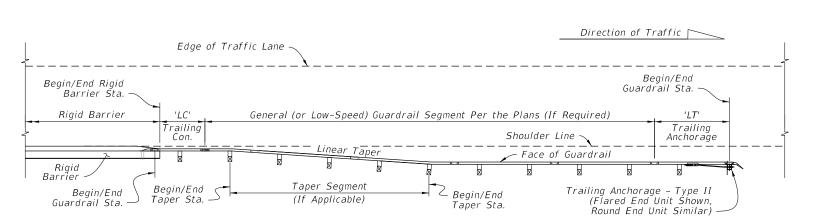
INDEX

Approach Terminal

SHEET

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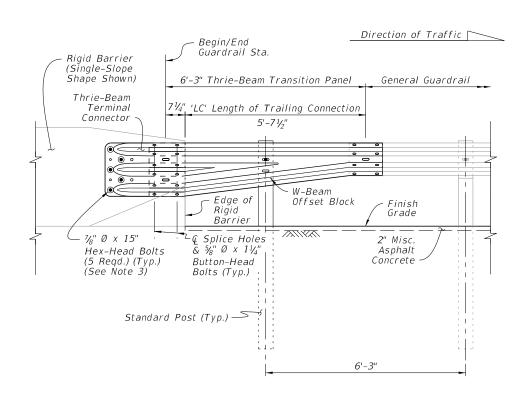


TYPE D TRAILING CONNECTION FROM RIGID BARRIER PLAN VIEW - MEDIAN OR OUTSIDE SHOULDER (Mirror Horiz. and/or Vert. for Opposite Direction and/or Side of Road)

NOTES:

DESCRIPTION:

- 1. See the applicable Notes on Sheet 17.
- 2. LENGTH OF TRAILING ANCHORAGE, 'LT': Install the Trailing Anchorage Type II as shown on Sheet 9, where called for in the plans.
- 3. THRIE-BEAM TERMINAL CONNECTOR: Install connector and bolts as shown on Sheet 15.
- 4. RIGID BARRIER SINGLE SLOPE END FACE: See Concrete Barrier Wall, Index 521-001, and Traffic Railing, Indexes 521-422 and 521-423, for details.



TRAILING END TRANSITION CONNECTION TO RIGID BARRIER - INSTALLED ELEVATION

> LAYOUT TO RIGID BARRIER -TRAILING ENDS

APPROACH ENDS WITH DOUBLE FACED GUARDRAIL

LAST **REVISION** 11/01/17

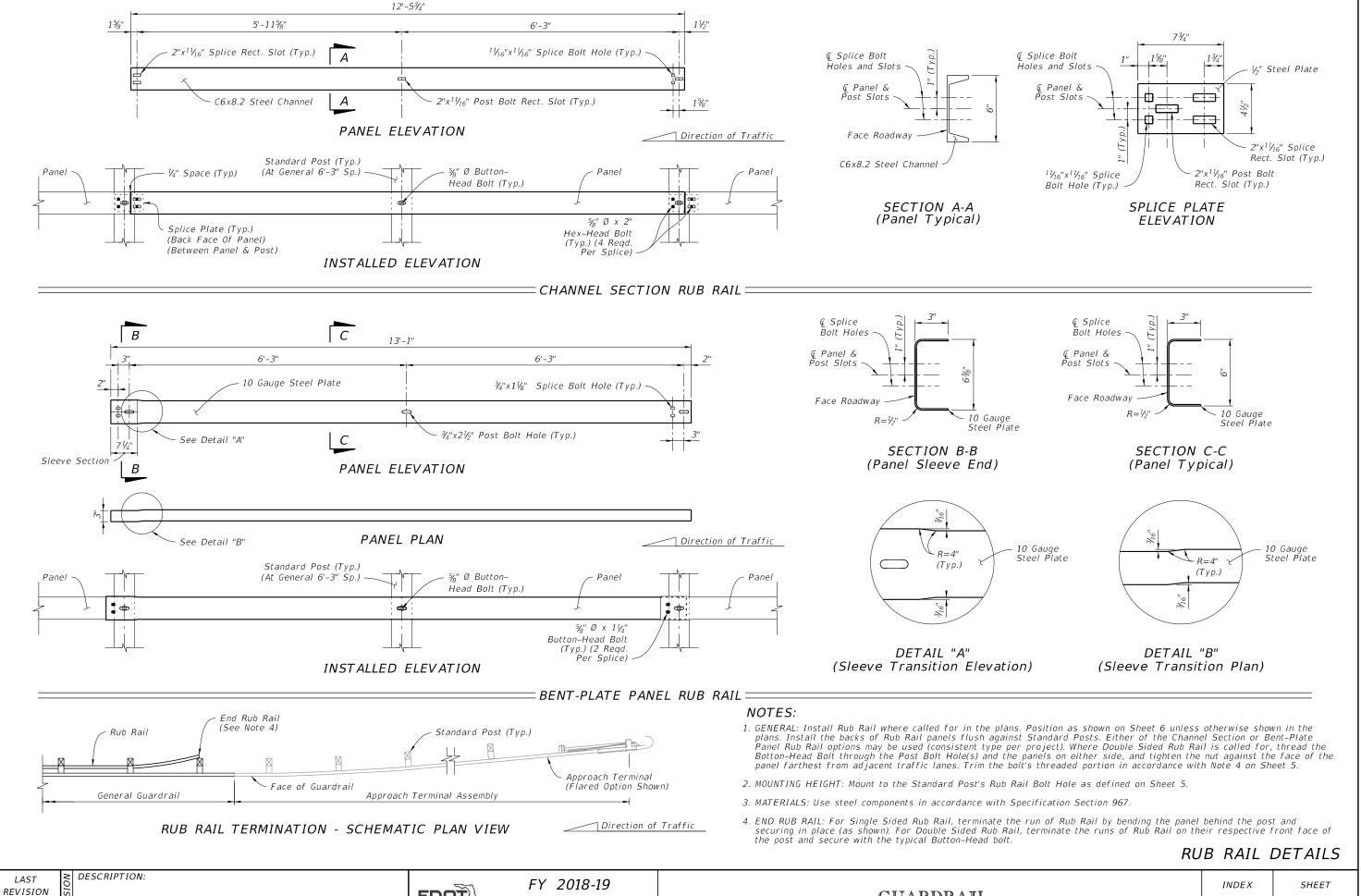
FDOT

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GUARDRAIL

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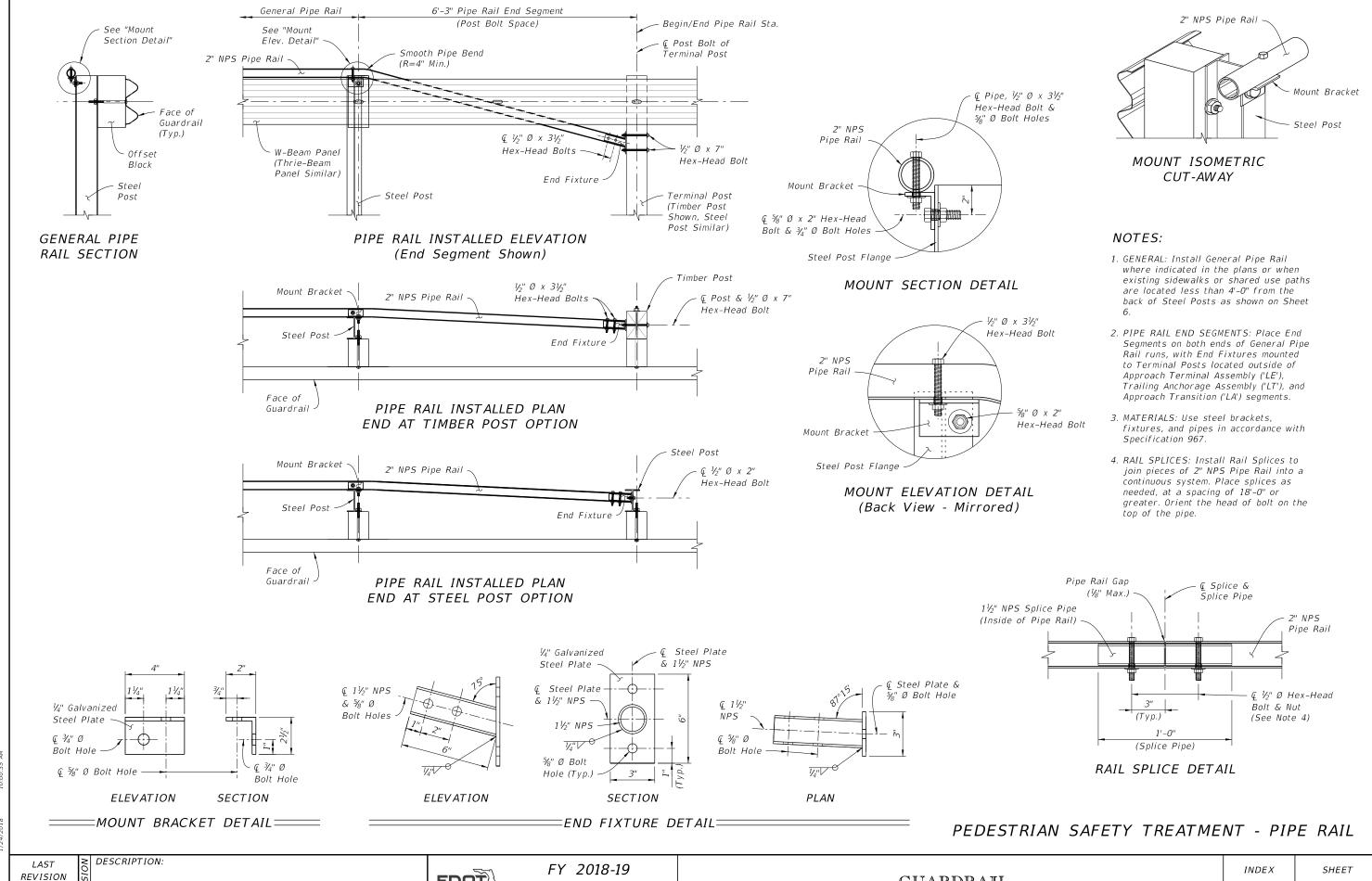


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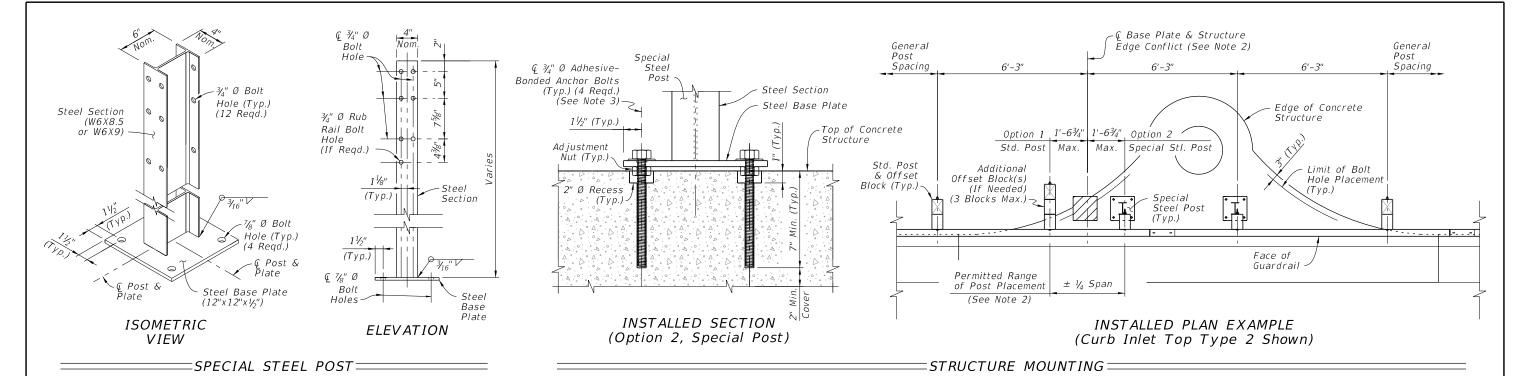
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STANDARD PLANS

GUARDRAIL

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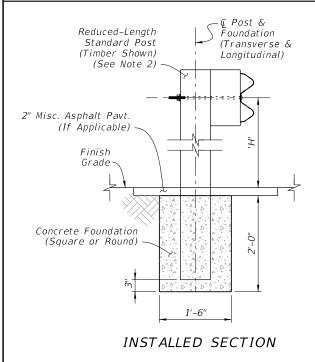
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NOTES:

- 1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) located atop culverts, inlets, pier footings, or similar concrete structures, a Special Steel Post may be substituted for a Standard Post. Install where shown in the plans and/or as-needed, in accordance with Specification Section 536.
- 2. EDGE CONFLICT: When a required post location causes an Edge Conflict with the structure, where the Steel Base Plate is not located entirely on the structure at least 3" from the Edge of Concrete, the longitudinal post location may be altered by up to 1'-6¾" (Quarter Span) from the original required spacing location to prevent the Edge Conflict. With the post location adjusted, use a Std. Post mounted in soil (Option 1) or a Special Steel Post with its Base Plate mounted entirely on the structure (Option 2). Maintain the original required spacing locations upstream and downstream
- 3. BASE PLATE MOUNT: Install Special Steel Posts as shown using steel Adhesive-Bonded Anchor Bolts in accordance with Specifications Section 536. Use 3/4" Hex-Head Bolts for structures less than 9" deep as defined in the Specification.
- 4. PANEL MOUNT TO ADJUSTED POST: Punch additional ¾"x2½" Post Bolt Slot(s) in the W-Beam or Thrie-Beam Panel only where needed to mount the panel to a post in an adjusted location. Meet the Panel Post Bolt Slots requirements of Specification Section 536.
- 5. MATERIALS: Use steel base plates in accordance with Specification Section 536.

SPECIAL STEEL POST FOR CONCRETE STRUCTURE MOUNT

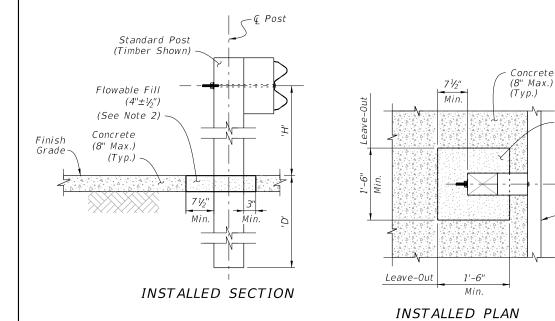


DESCRIPTION:

NOTES:

ENCASED POST FOR SHALLOW MOUNT

- 1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) conflicting with underground utilities or other underground obstructions, an Encased Post may be used where a 2'-0" depth will avoid the conflict. Install where shown in the plans and/or as-needed, in accordance with Specification Section 536.
- 2. REDUCED-LENGTH STANDARD POST: Use a Standard Post with reduced Length such that the Panel Height 'H' is maintained while the post bottom terminates 3" from the bottom of the Concrete Foundation. Typically, the Post Length 'L' is 4'-7" for W-Beam Guardrail.
- 3. FOUNDATION: Use non-reinforced Class NS Concrete material in accordance with Specification Section 347. After casting the concrete, ensure the surrounding soil material is completely backfilled and tamped to provide full passive resistance.
- 4. LIMIT: Encased Posts are not permitted for consecutive posts unless otherwise shown in the plans.



NOTES:

Flowable Fill

(See Note 2)

Face of

Guardrail

@ Post &

Flowable Fill

(4"±1/3")

1. INSTALLATION: When the construction of Guardrail at the required post spacing results in post(s) placed within a concrete surface (typically a sidewalk), use a Frangible Leave-Out around the post base as shown. Install where shown in the plans and/or as-needed, in accordance with Specification Section 536.

For the required 1'-6" x 1'-6" Leave-Out, smoothly cut the existing concrete surface or form-up the square shape when an application has new surrounding concrete

Ensure Flowable Fill surface is smooth and even with the adjacent concrete

2. MATERIALS: Use Non-Excavatable Flowable Fill in accordance with Specification Section 121, not to exceed 150 psi

FRANGIBLE LEAVE-OUT FOR CONCRETE SURFACE MOUNT

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FDOT

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GUARDRAIL

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- 2. MATERIALS: Use materials of the size and type defined for Barrier Delineators in Specifications Section 993.
- 3. COLOR: Use either white or yellow retroreflective sheeting to match the color of the nearest lane's edgeline.
- 4. MOUNT LOCATIONS: Mount Barrier Delineators atop posts as shown, starting with Post (3) of Approach Terminals and incrementally increasing spacing towards the downstream direction. Install the Barrier Delineators at the following

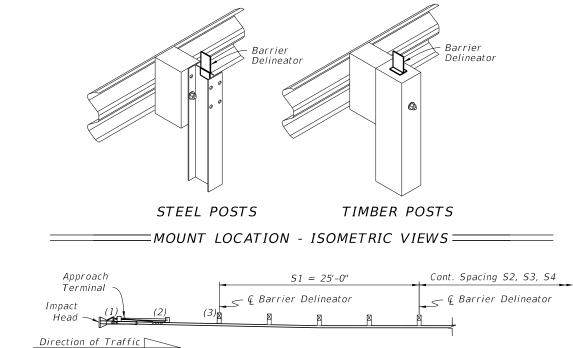
 $S1 = 25' \times 1 \ Space$

 $S2 = 50' \times 1 \ Space$ $53 = 75' \times 1 \text{ Space}$

 $S4 = 100' \times for$ the Remaining Run

Additionally, place a Barrier Delineator on Post (2) of the Trailing Anchorage or on the post nearest the Rigid Barrier.

5. MEDIAN GUARDRAIL: Install retroreflective sheeting on both sides of the barrier delineator for Guardrail on medians.



MOUNT LOCATION - PLAN VIEW =

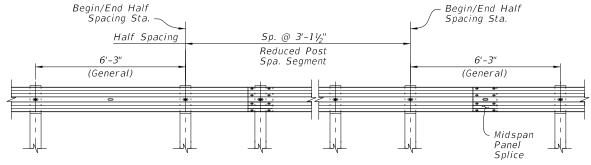
BARRIER DELINEATORS

NOTES:

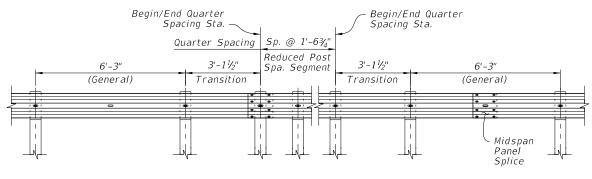
1. INSTALLATION: Work these details with the plans, where Stationing for Begin/End Half Spacing and Begin/End Quarter Spacing are indicated if required.

Where the Begin/End Stations indicated in the plans do not correspond exactly to post locations in construction, extend the Reduced Post Spacing segment to the nearest post(s) before the Begin Station and/or after the End Station called for.

- 2. PANEL SPLICES: Midspan Panel Splices are not required in Transition and Reduced Post Spacing segments, however they are required for General segments. To place midspan splices in General segments, use one Non-General panel length (9'-41/2" or 15'-71/2") or add an additional Transition spaced post where required.
- 3. LOW-SPEED GUARDRAIL: For Reduced Post Spacing with Low-Speed Guardrail (12'-6" post spacing), the Reduced Spacing pattern requires a 6'-3" space between the 12'-6" and 3'-11/3"
- 4. PANEL POST BOLT SLOTS: For Quarter Spacing configurations, punch additional 3/4"x21/2" Post Bolt Slots in the panels only where required for mounting and in accrodance with Specification Section 536.

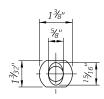


DETAIL 'S' - HALF SPACING ELEVATION (AS REQ'D. PER THE PLANS)

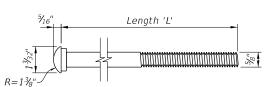


DETAIL 'S' - QUARTER SPACING ELEVATION (AS REQ'D. PER THE PLANS)

REDUCED POST SPACING FOR HAZARDS





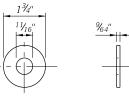


ELEVATION OPTION 1

ELEVATION OPTION 2

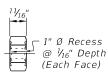
PROFILE (Option 1 Shown)

BUTTON-HEAD BOLT =





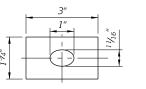




ELEVATION PROFILE ==WASHER ====



===HEX-NUT =====





ELEVATION

PROFILE

 \equiv RECTANGULAR WASHER \equiv (For Type II, CRT, & Terminal Connectors Where Shown -Install Over Panel Face)

BUTTON-HEAD BOLT LENGTHS:

Application(s):	Length 'L':	Min. Thread Length:
Panel Splice	1 1/4"	Full Length
Steel Post Mount – Single Faced Guardrail	10"	4"
Timber Post Mount - Single Faced Guardrail	18"	4"
Steel or Timber Post Mount - Double Faced Guardrail	25"	4"
Modified Thrie-Beam Panel / Terminal Connector Splice	2"	Full Length

NOTES:

- 1. Use nuts, bolts, and washers in accordance with Specification Section 967.
- 2. For Steel Posts with Double Faced Guardrail, the single 25" Length bolt (one bolt thru both post flanges) may be replaced with two 10" Length bolts (one bolt per post flange).
- 3. Use bolts listed in Table 2 in corresponding locations shown in this Index.

5/8" BUTTON-HEAD BOLT SYSTEM

FDOT

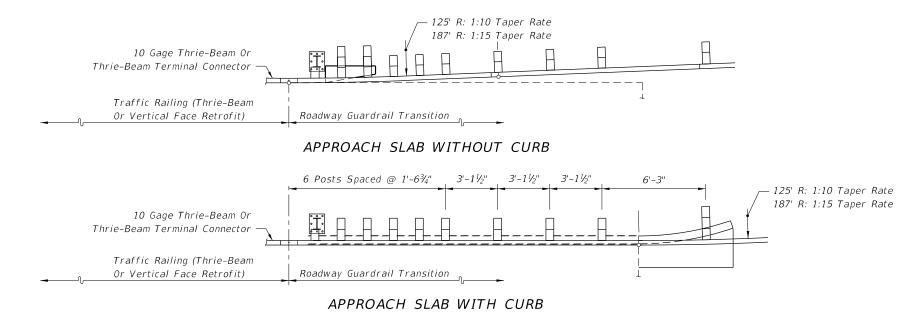
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SPECIAL STEEL POST FOR ROADWAY THRIE-BEAM TRANSITIONS TO BRIDGE TRAFFIC RAILING RETROFITS



Longitudinal Location Of Transition Blocks And Curb End Flares Will Vary With Scheme Type

PARTIAL PLAN VIEWS

GENERAL NOTES

- 1. This index provides guardrail transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 26 apply to bridges with retrofitted traffic railings, (Sheet 26 shows the trailing end guardrail connections). Sheet 27 applies to bridges with safety shaped traffic railing. Construct the guardrail transitions and connections where shown in the plans.
- 2. For trailing end guardrail connections for existing bridges with either Vertical Face Retrofits or Safety Shape Traffic Railing, see the Trailing End Transition Connection to Rigid Barrier detail shown in Index 536-001. Likewise, for miscellaneous guardrail construction details that are not provided in this Index, refer to Index 536-001.

NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- 1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.
- 2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to curb blunt ends are not in place.
- 3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated in accordance with Specification 967.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification 937 and be installed in accordance with Specification 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

- 5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 15 through 18 and the flag notation on Sheet 26.
- 6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

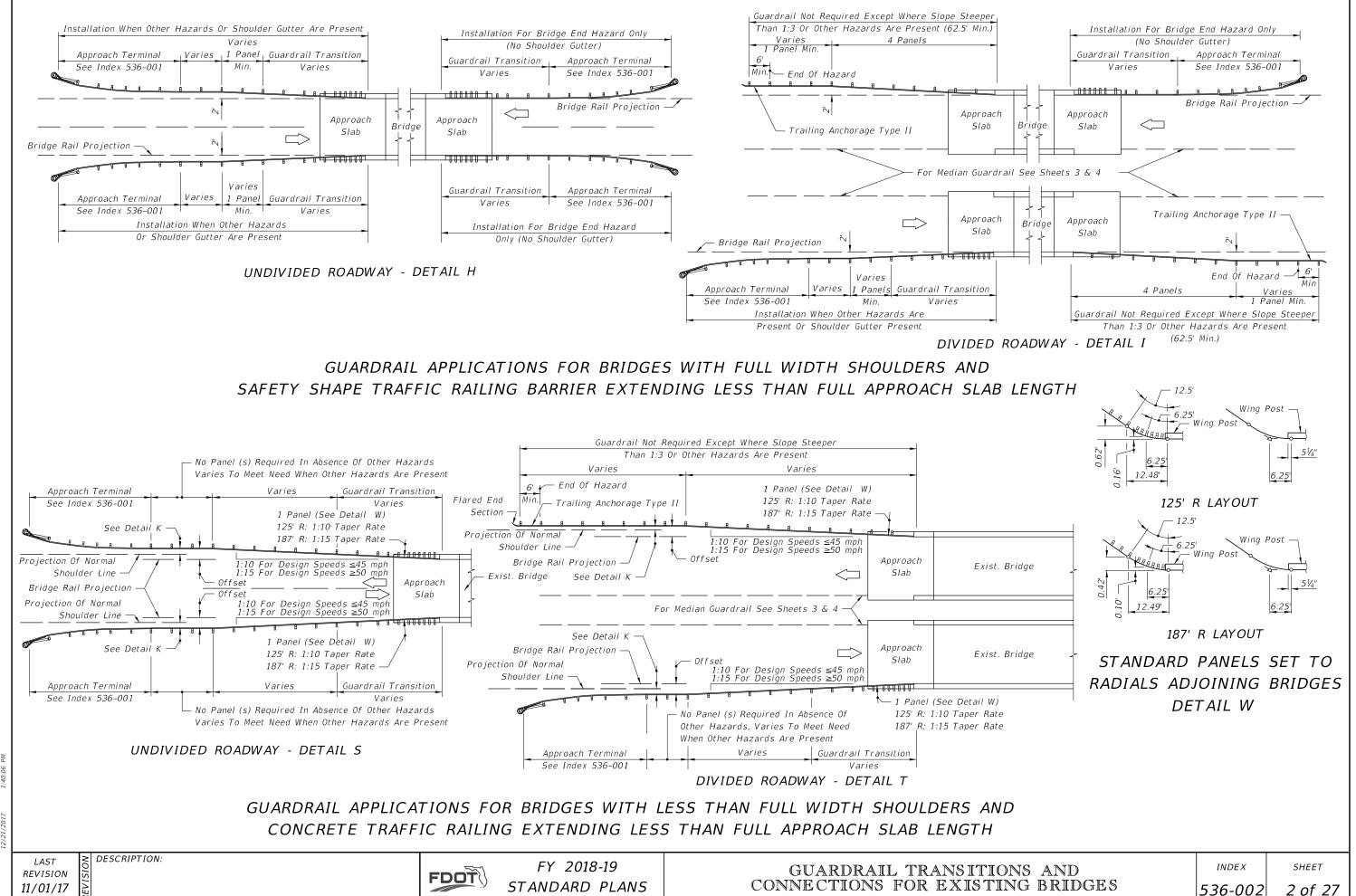
GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT

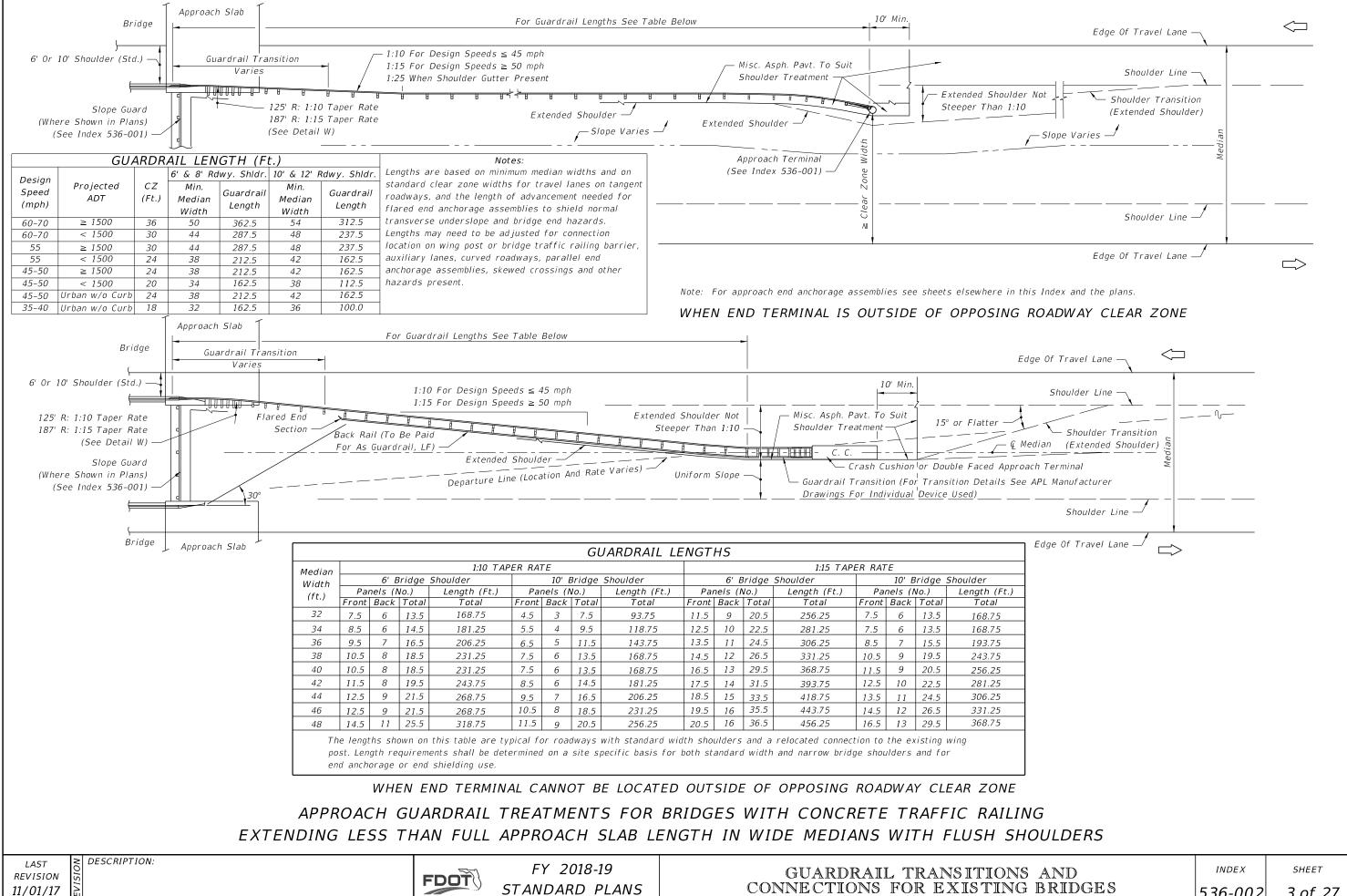
LAST REVISION 11/01/17

DESCRIPTION:



536-002

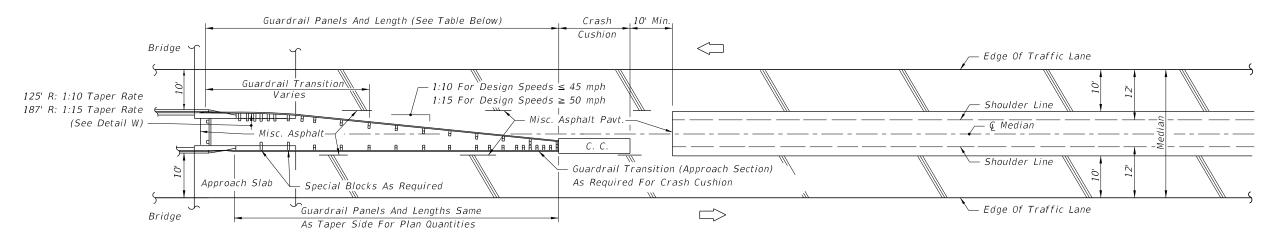




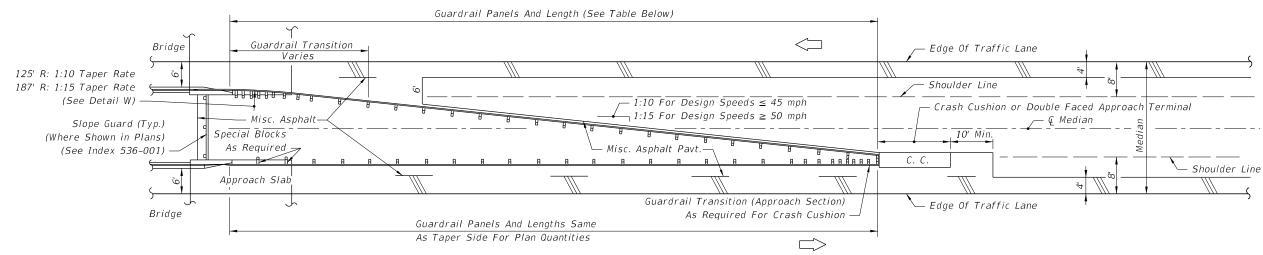
STANDARD PLANS

CONNECTIONS FOR EXISTING BRIDGES

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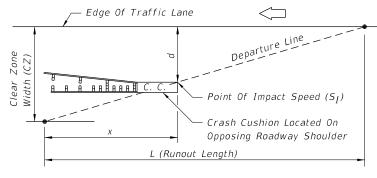


MEDIANS WITH 10' BRIDGE SHOULDERS



MEDIANS WITH 6' BRIDGE SHOULDERS

Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.



Speed (S_I) For Determining Crash Cushion Size: $S_I = \frac{x}{L} (Design Speed) = \left[\frac{(CZ - d)}{CZ}\right] Design Speed$

SIZING CRASH CUSHIONS LOCATED ON OPPOSING ROADWAY SHOULDERS

	GUARDRAIL LENGTHS							
MEDIAN	6' BRIDGE SHOULDERS			10' BRIDGE SHOULDERS				
WIDTH	1:10 TAPER RATE		1:15 TAPER RATE		1:10 TAPER RATE		1:15 TAPER RATE	
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	12.5	156.25	18.5	231.25	6.5	81.25	9.5	118.75
28	11.5	143.75	16.5	206.25	5.5	68.75	7.5	93.75
26	9.5	118.75	14.5	181.25	5.5*	68.75	5.5*	68.75
24	8.5	106.25	11.5	143.75	5.5*	68.75	5.5*	68.75

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds $(S_1$'s) along the runouts from the approach roadways; however, when calculated speeds (S_I) 's) are less than 30 mph crash cushions shall be no less in size than for 30 mph; see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see * below.

*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced quardrail must have a length of five (5) or more panels.

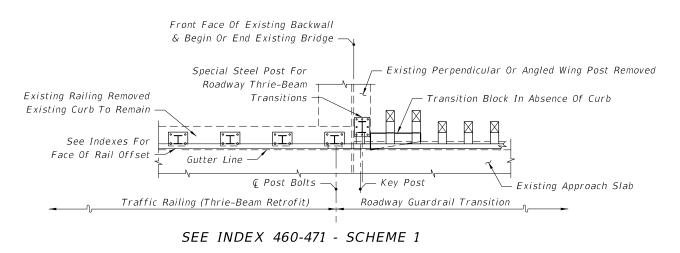
APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH CONCRETE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

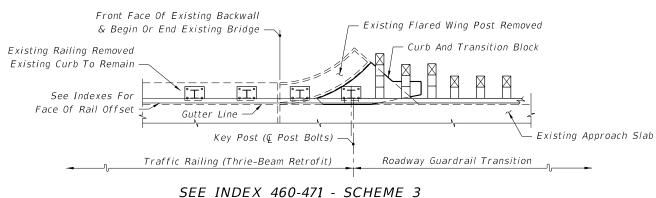
REVISION 11/01/17

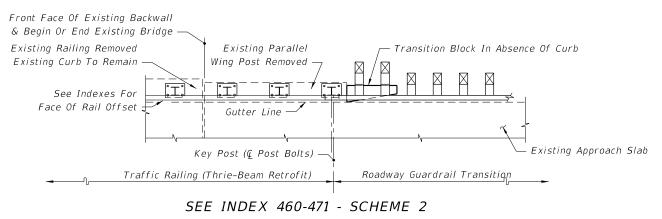
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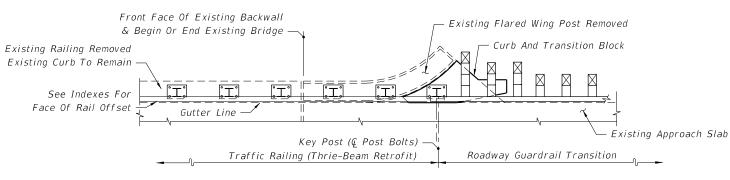
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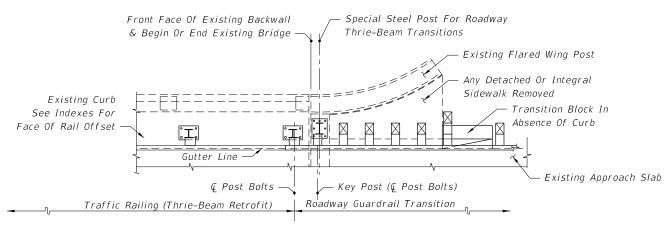
SEE INDEX 460-471 - SCHEME 3

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

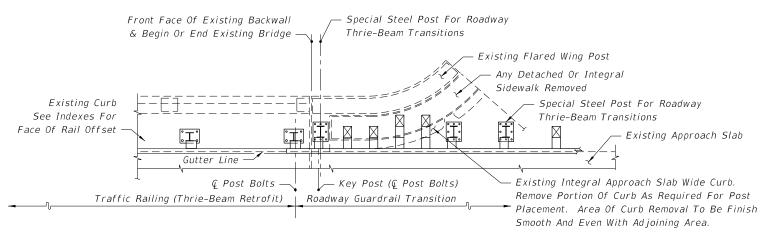
REVISION 11/01/17

DESCRIPTION:

SEE INDEXES 460-472 & 460-475 - SCHEME 2



SEE INDEXES 460-472 & 460-475 - SCHEME 2



SEE INDEXES 460-472 & 460-475 - SCHEME 2

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS
FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

10/23/2017 1:28:06

LAST REVISION 11/01/17

DESCRIPTION:

Front Face Of Existing Backwall

Gutter Line

Traffic Railing (Thrie-Beam Retrofit)

Existing Curb

See Indexes For

Face Of Rail Offset

& Begin Or End Existing Bridge —

© Post Bolts -

SEE INDEXES 460-472 & 460-475 - SCHEME 1

- Existing Perpendicular Or Angled Wing Post

Transition Block In Absence Of Curb

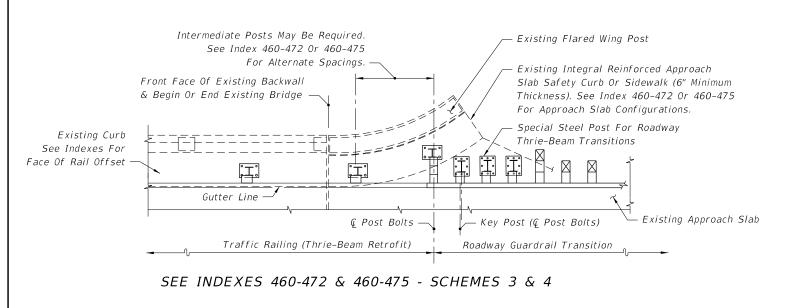
- Existing Approach Slab

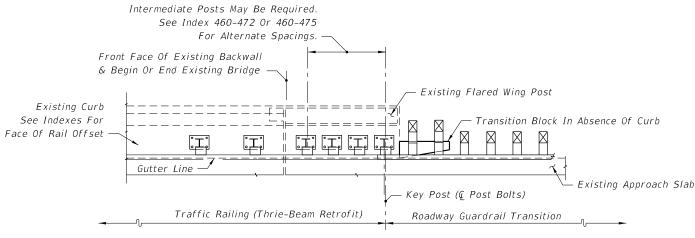
Special Steel Post For Roadway

Thrie-Beam Transitions

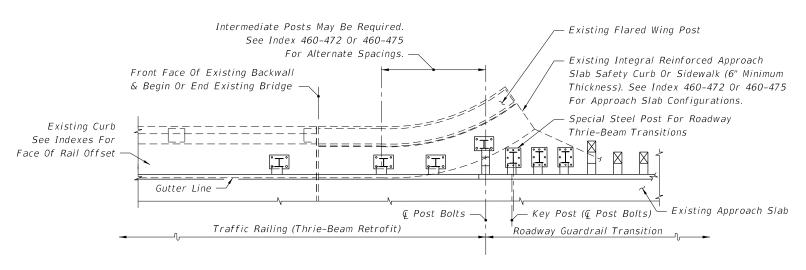
– Key Post (& Post Bolts)

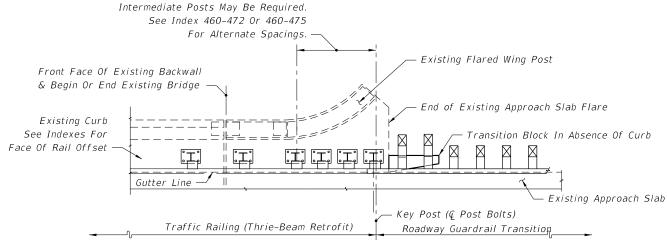
Roadway Guardrail Transition





SEE INDEXES 460-472 & 460-475 - SCHEMES 5 & 6





SEE INDEXES 460-472 & 460-475 - SCHEMES 3 & 4

SEE INDEXES 460-472 & 460-475 - SCHEMES 5 & 6

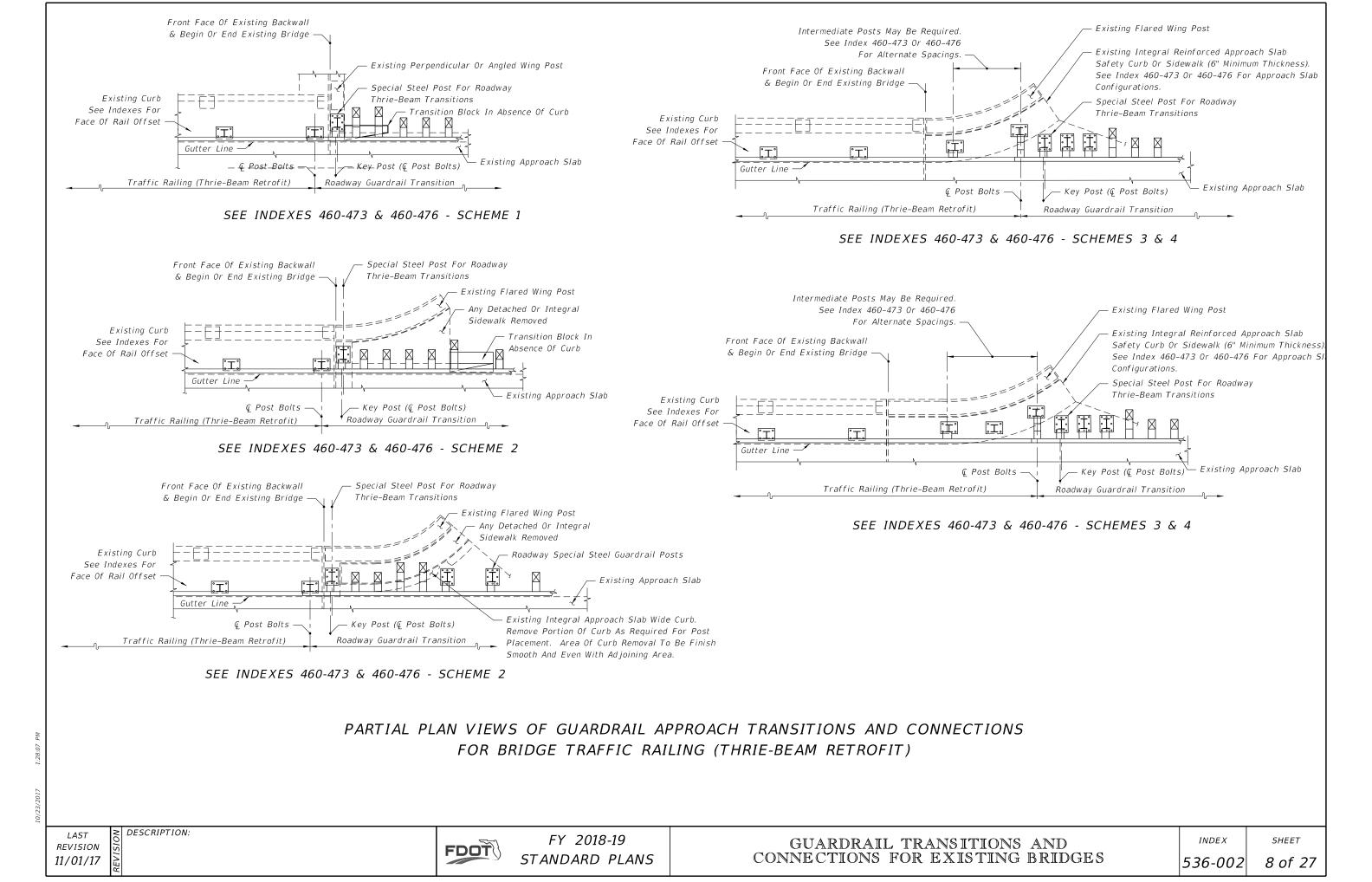
PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS
FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

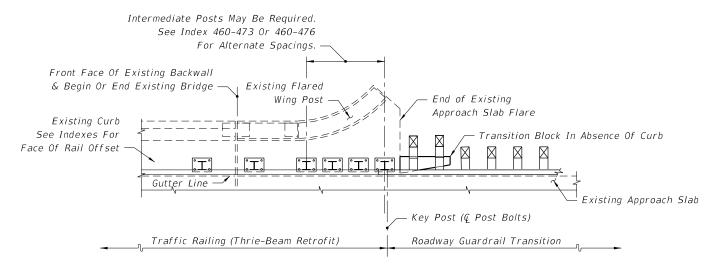
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LAST REVISION 11/01/17

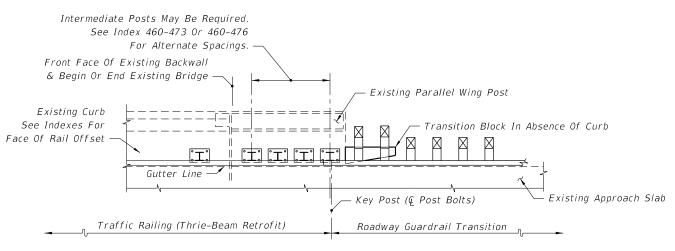
DESCRIPTION:

FDOT





SEE INDEXES 460-473 & 460-476 - SCHEMES 5 & 6

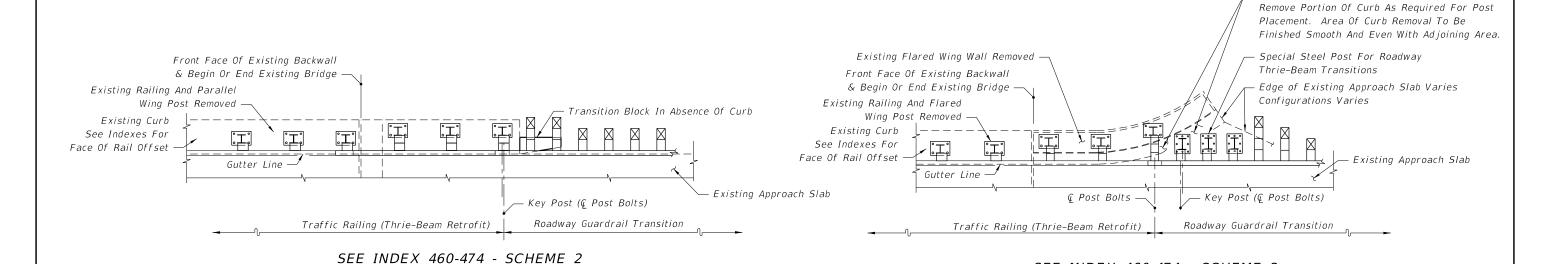


SEE INDEXES 460-473 & 460-476 - SCHEMES 5 & 6

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

DESCRIPTION:

SEE INDEX 460-474 - SCHEME 1



PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

10/23/2017

LAST REVISION 11/01/17

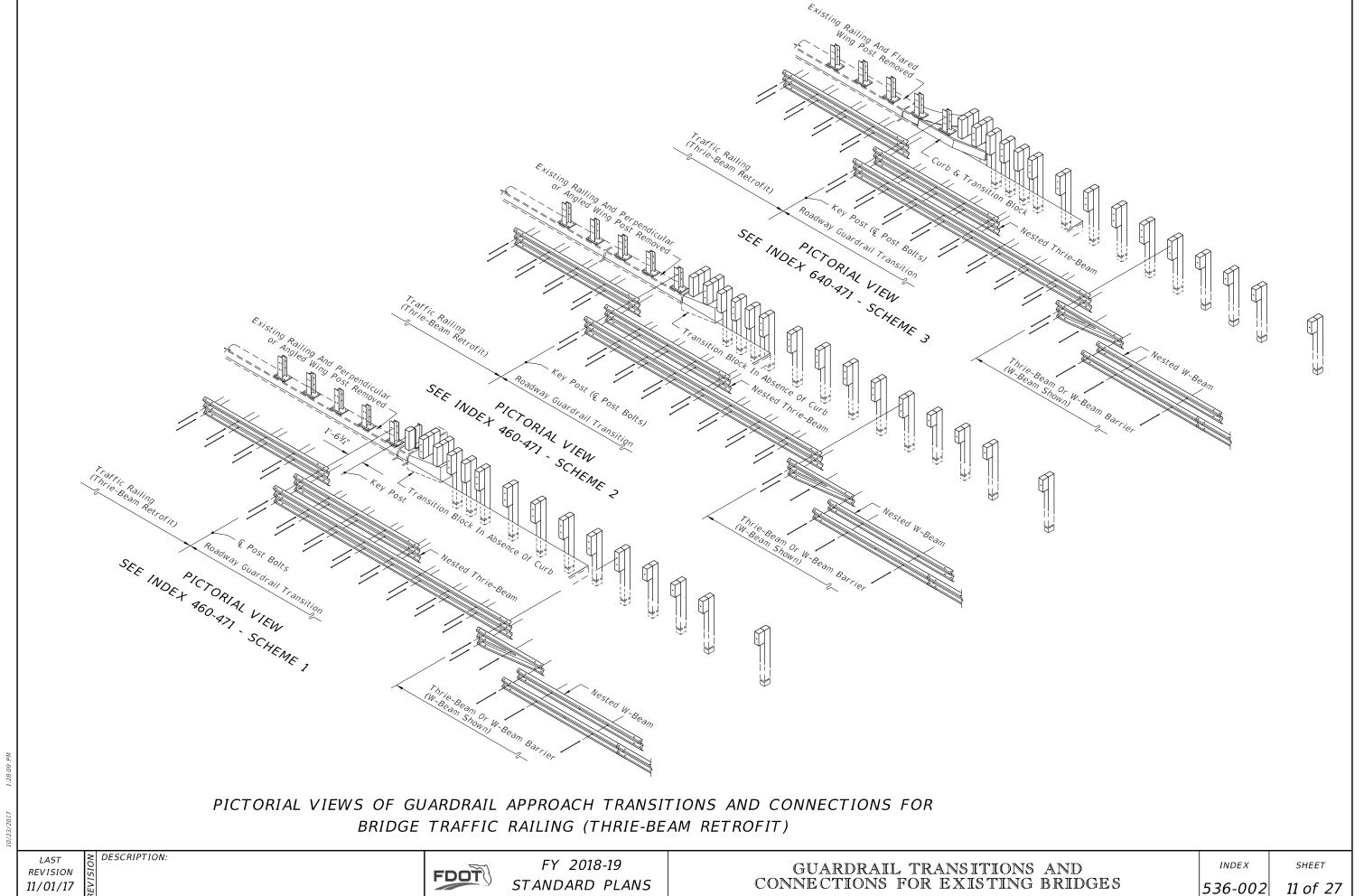
DESCRIPTION:

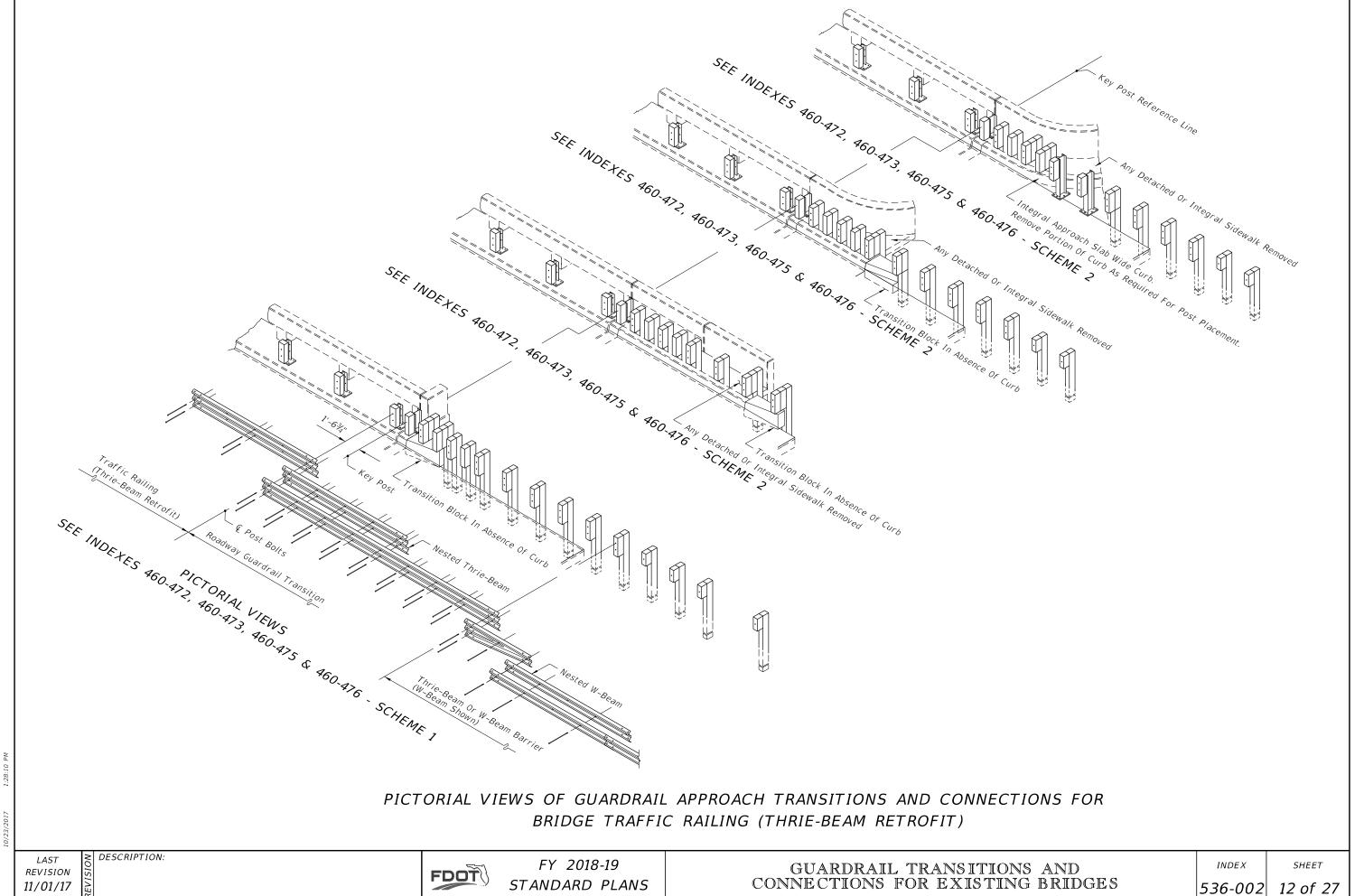
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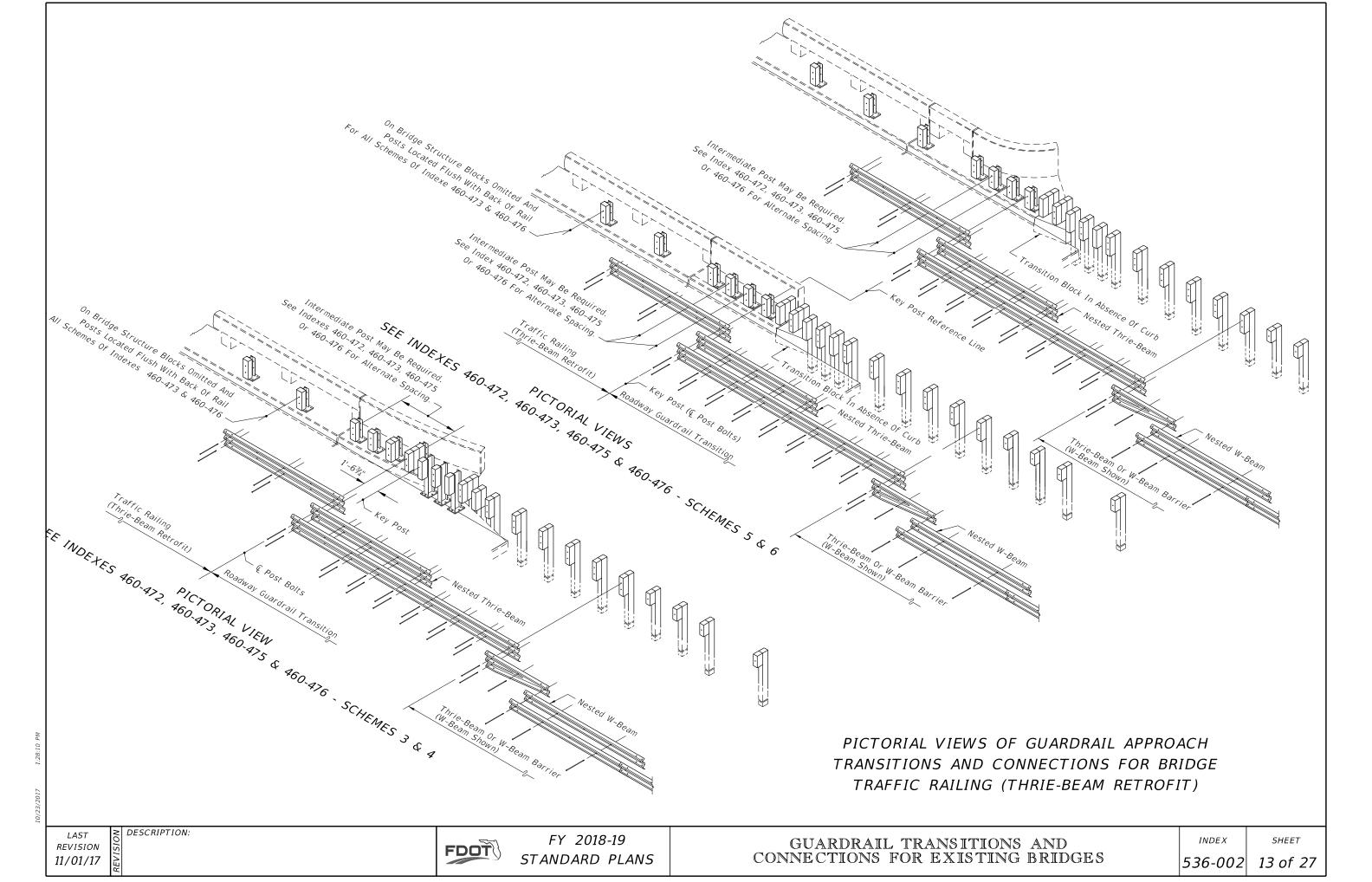
SEE INDEX 460-474 - SCHEME 3

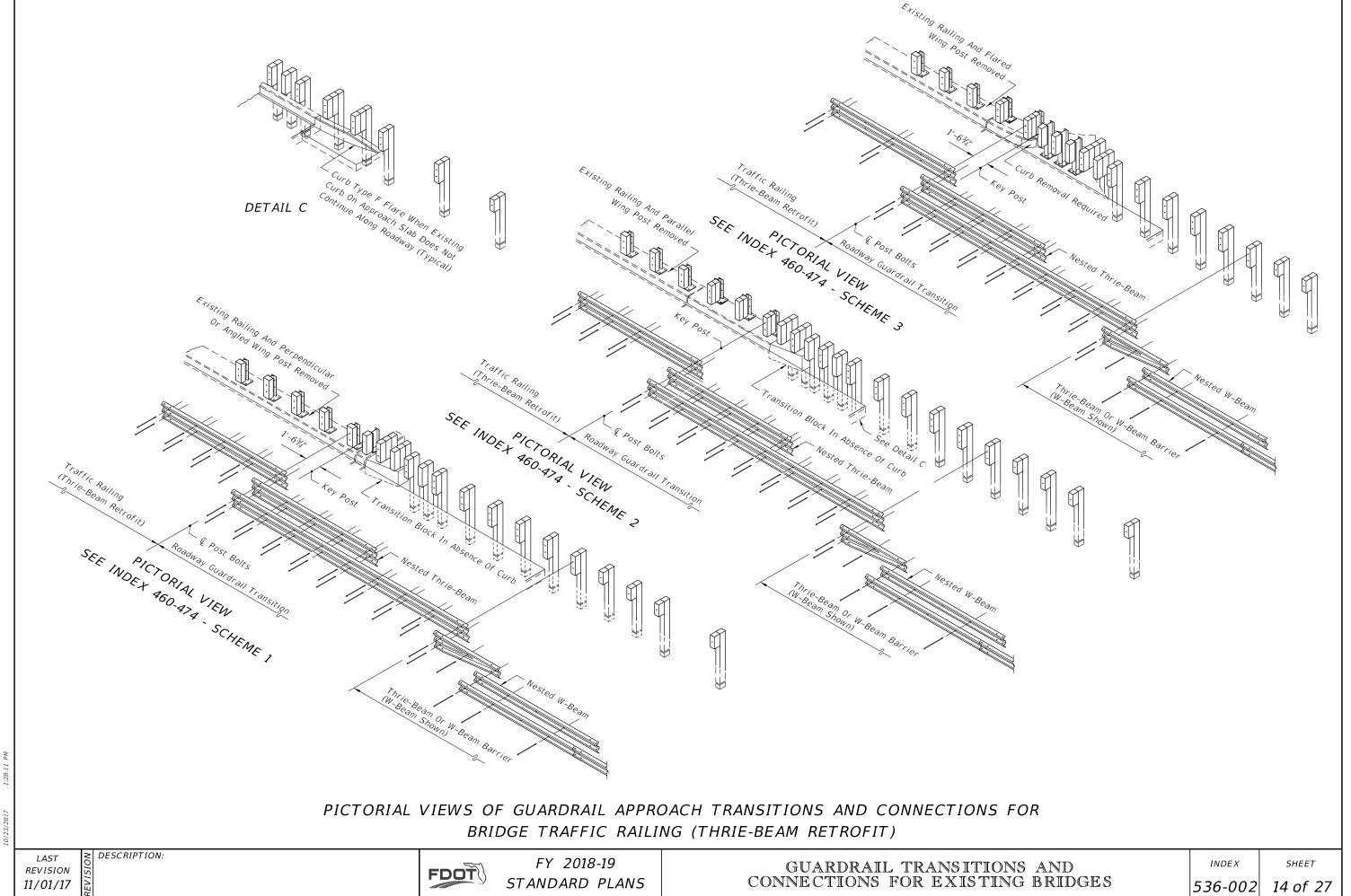
SEE INDEX 460-474 - SCHEME 3

Existing Integral Approach Slab Wide Curb.









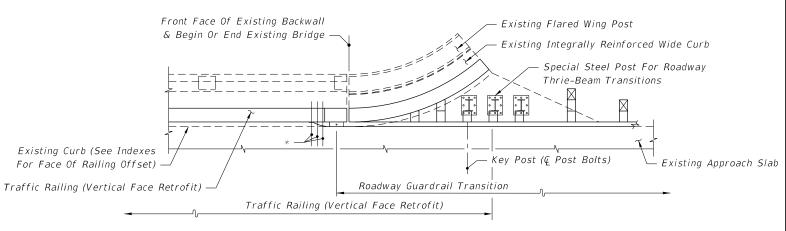
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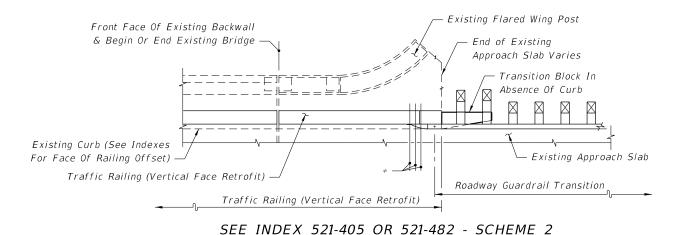
STANDARD PLANS

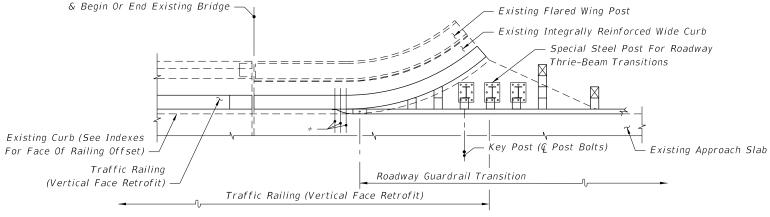
11/01/17

SEE INDEX 521-405 OR 521-482 - SCHEME 2



SEE INDEX 521-405 OR 521-482 - SCHEME 3





SEE INDEX 521-405 OR 521-482 - SCHEME 3

Note:

*21" x 12" x $\frac{5}{8}$ " Thrie-Beam Terminal Connector Plate (Back-Up Plate), And $\frac{7}{6}$ " Ø x 12" Long HS Hex Bolts And Nuts (5 Reqd.) With $\frac{2}{4}$ " OD Plain Round Washers Under Heads And Nuts

Front Face Of Existing Backwall

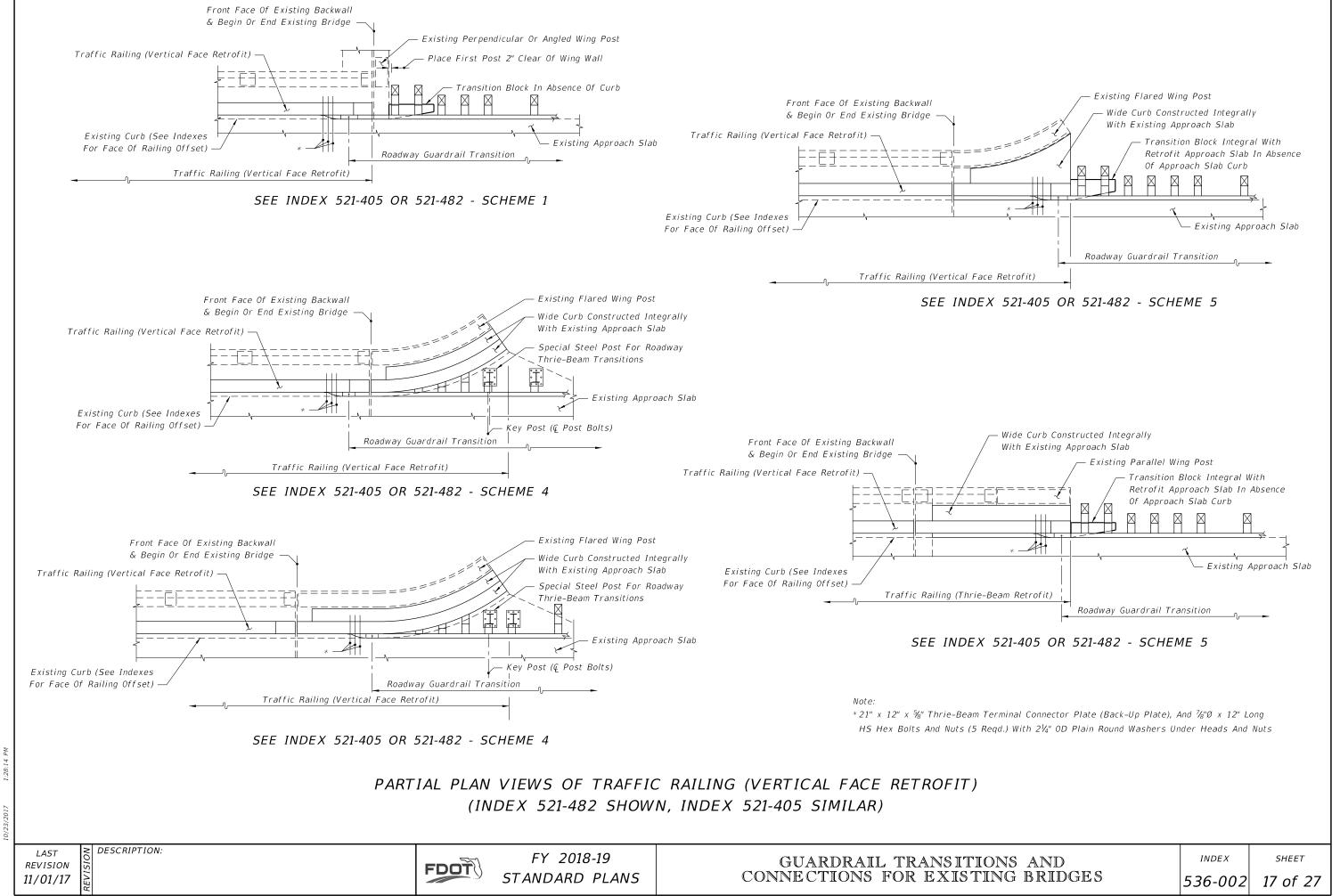
PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)
(INDEX 521-482 SHOWN, INDEX 521-405 SIMILAR)

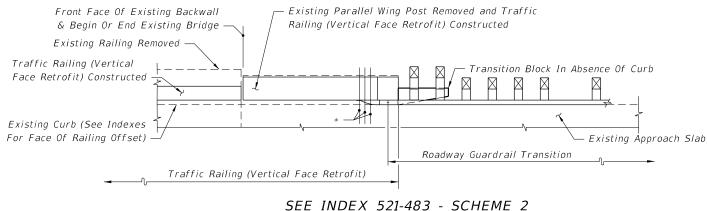
LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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Front Face Of Existing Backwall & Begin Or End Existing Bridge Existing Flared Wing Post Removed And Traffic Railing (Vertical Face Retrofit) Constructed. Existing Railing Removed And Special Steel Post For Roadway Traffic Railing (Vertical Thrie-Beam Transitions Face Retrofit) Constructed Edge of Approach Slab Varies See Structures Index 460-474 For Approach Slab Configurations. Existing Curb (See Indexes For Face Of Railing Offset) Key Post (© Post Bolts) -Existing Approach Slab Roadway Guardrail Transition Traffic Railing (Vertical Face Retrofit) SEE INDEX 521-483 - SCHEME 3

*21" x 12" x %" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And ½"Ø HS Hex Bolts And Nuts (12" Long For Scheme 1 And Length To Fit For Schemes 2 And 3) (5 Reqd.) With 21/4" OD Plain Round Washers Under Heads And Nuts

PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)

REVISION 11/01/17

DESCRIPTION:

FDOT

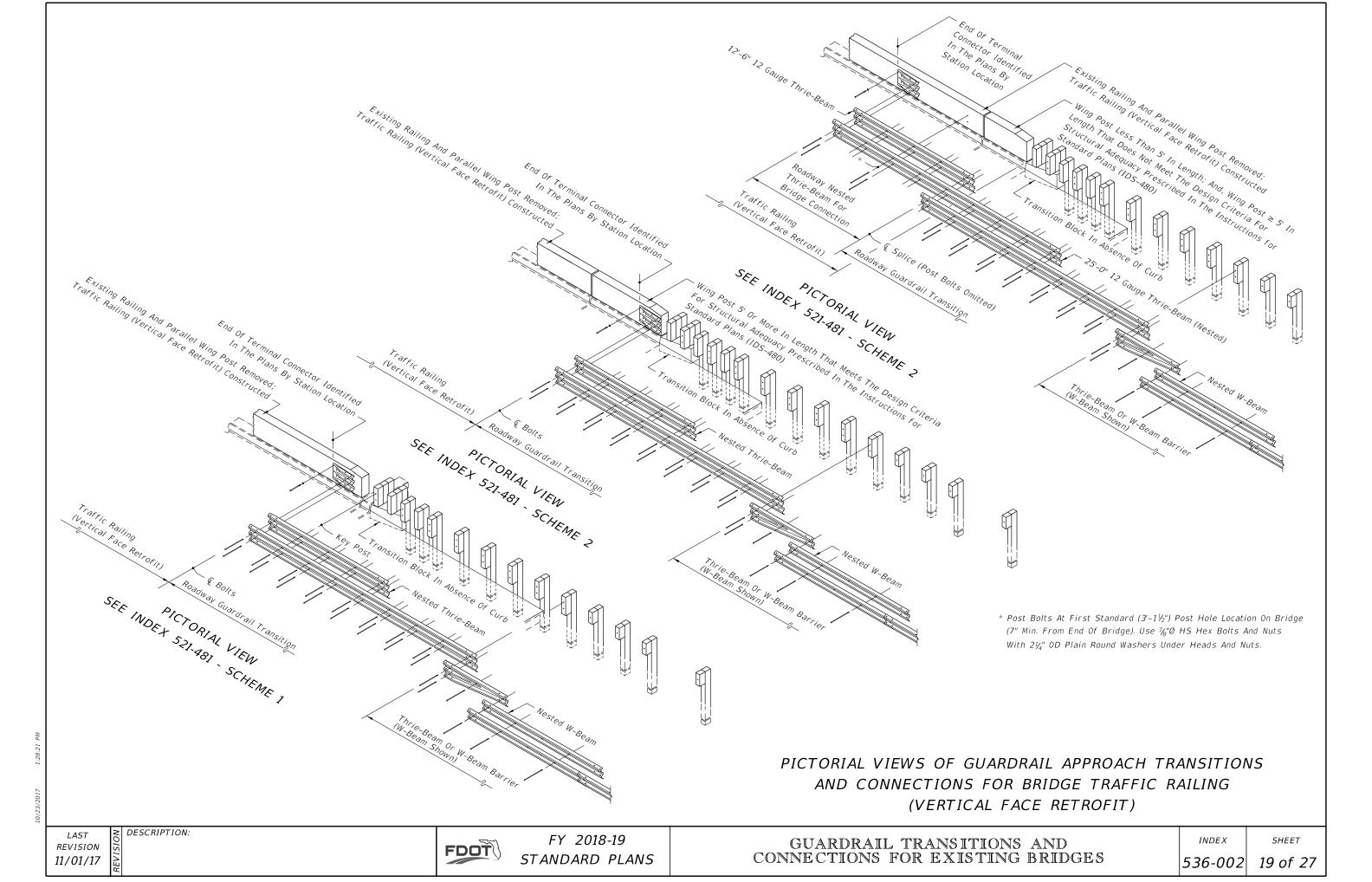
FY 2018-19 STANDARD PLANS

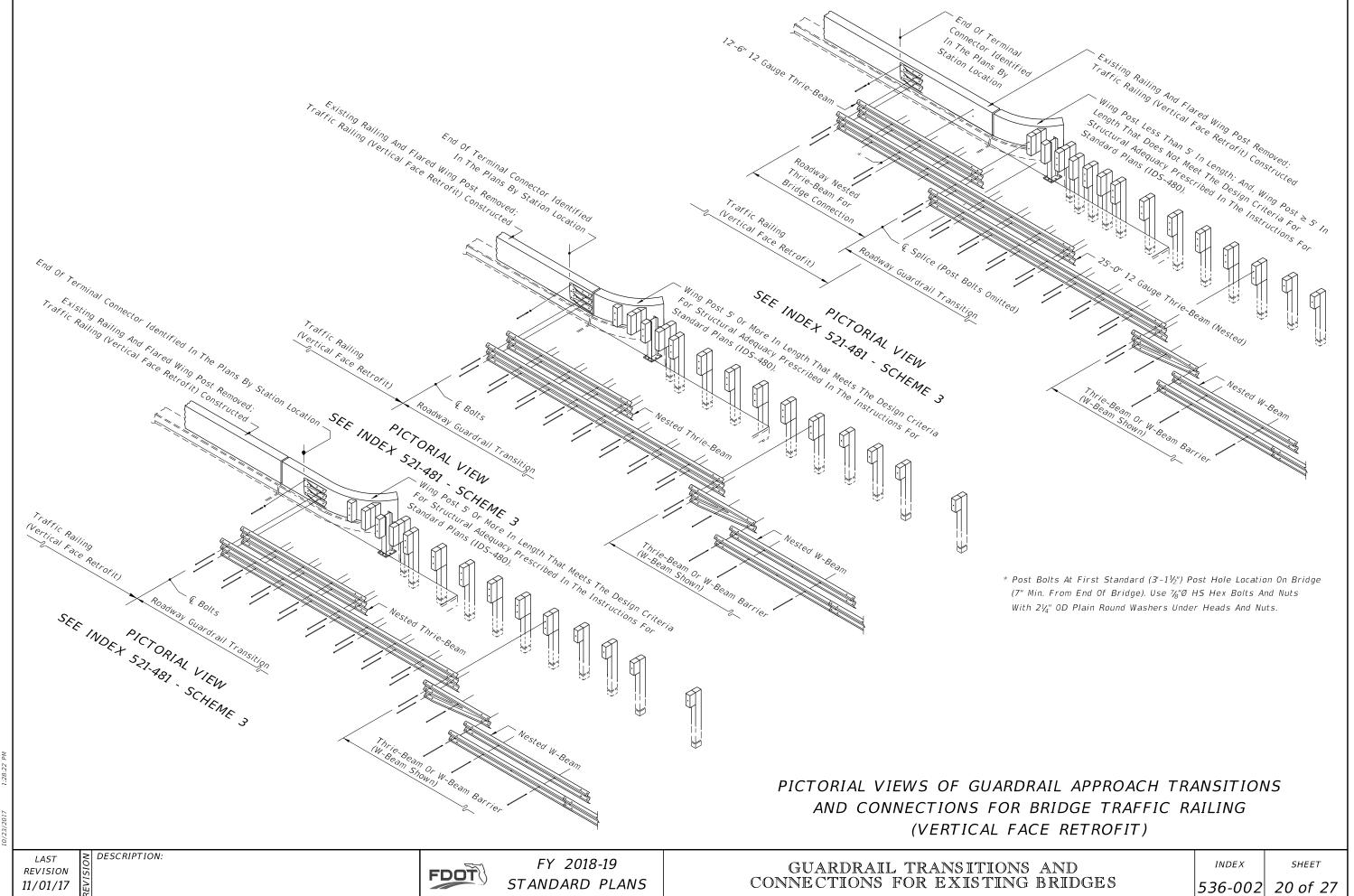
GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

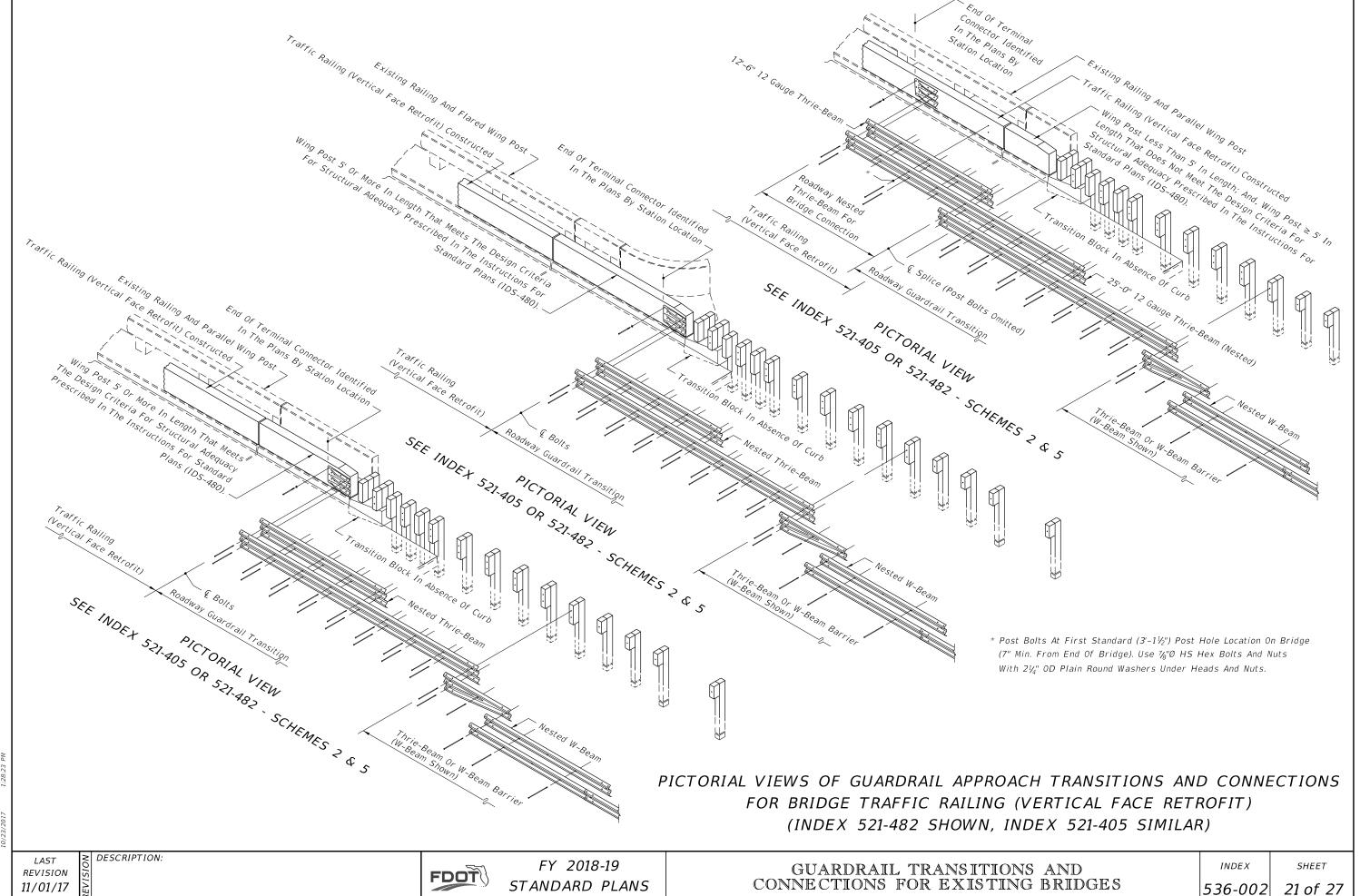
INDEX

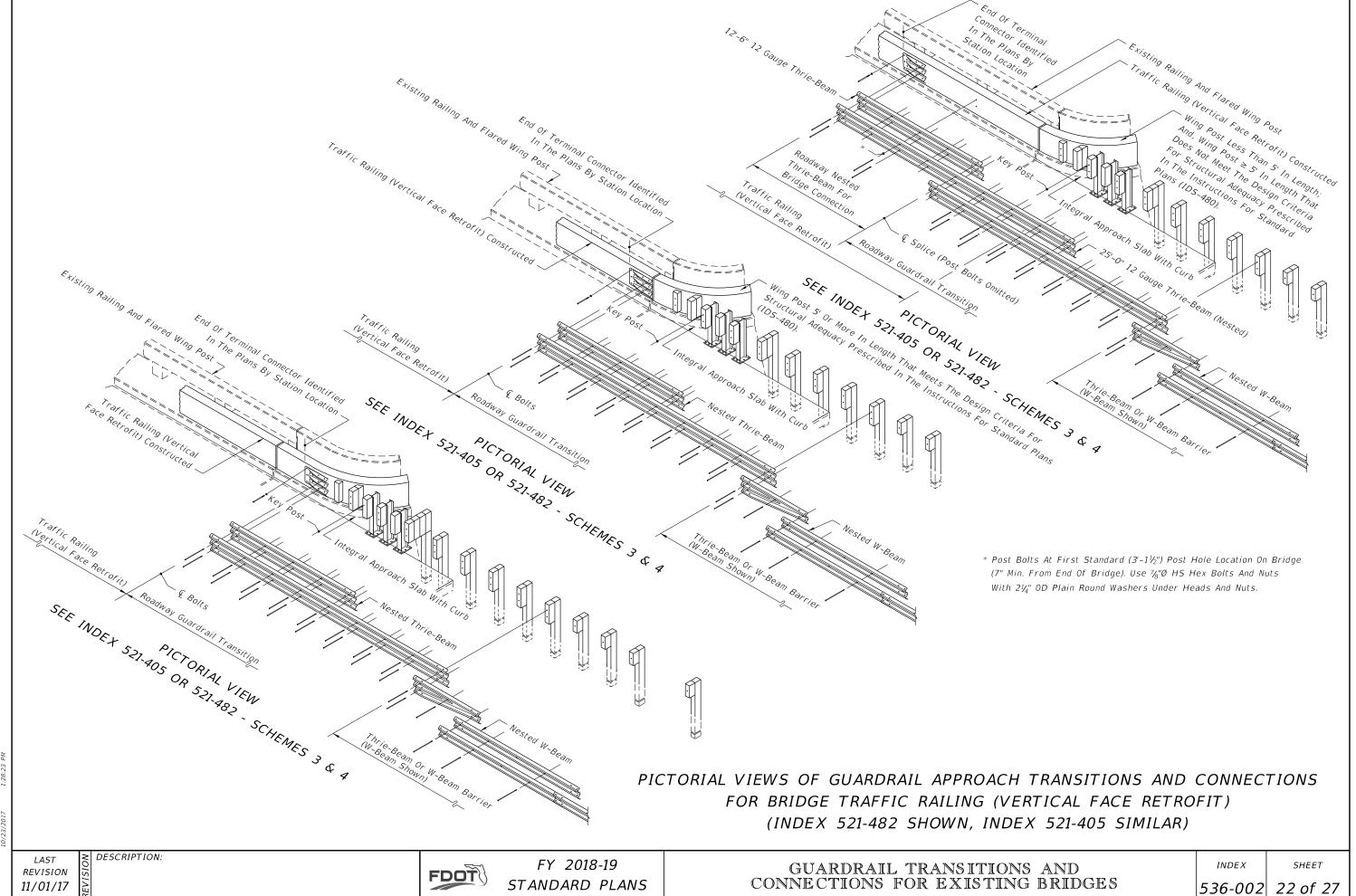
SHEET

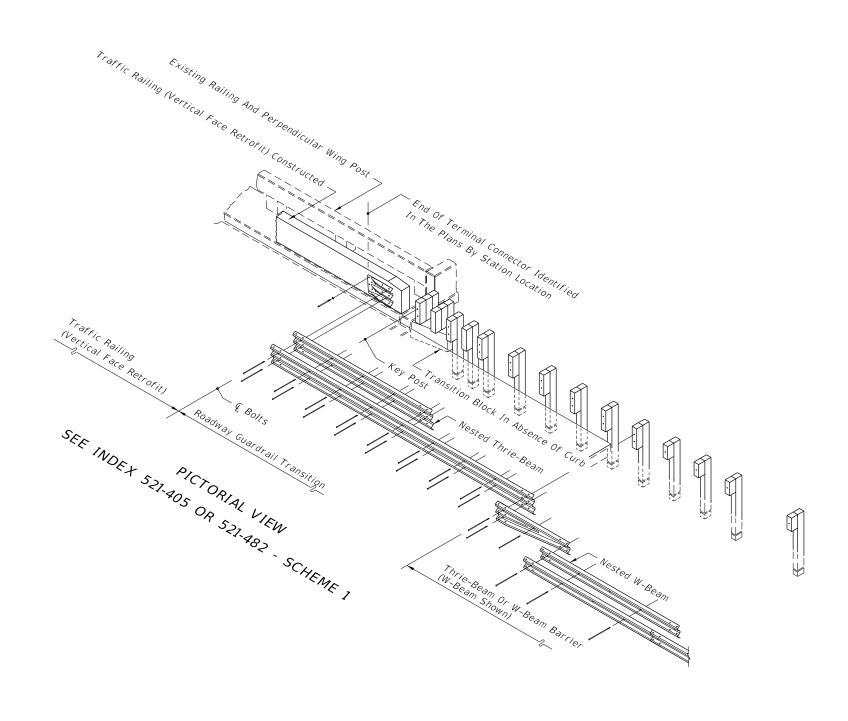
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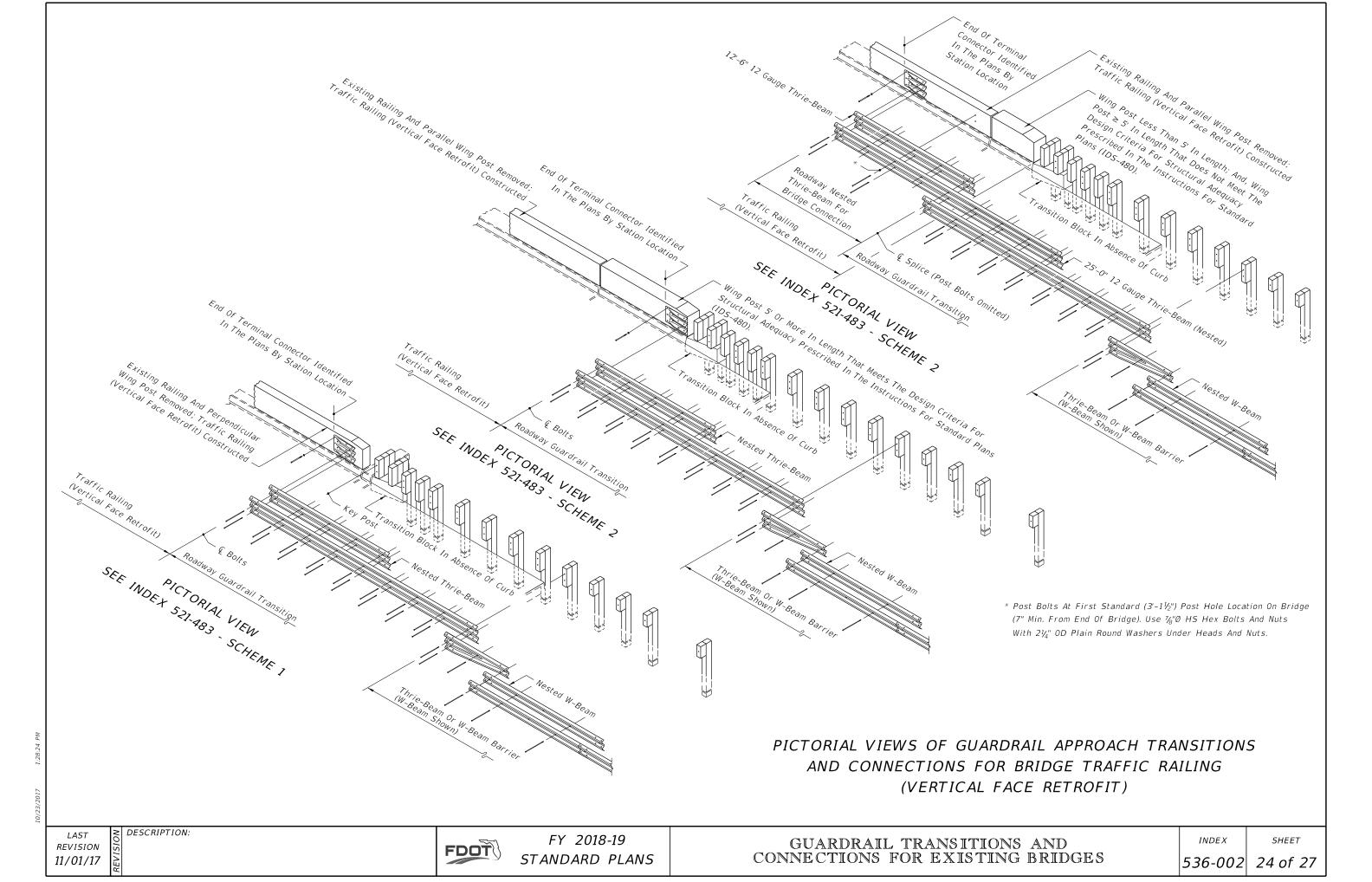


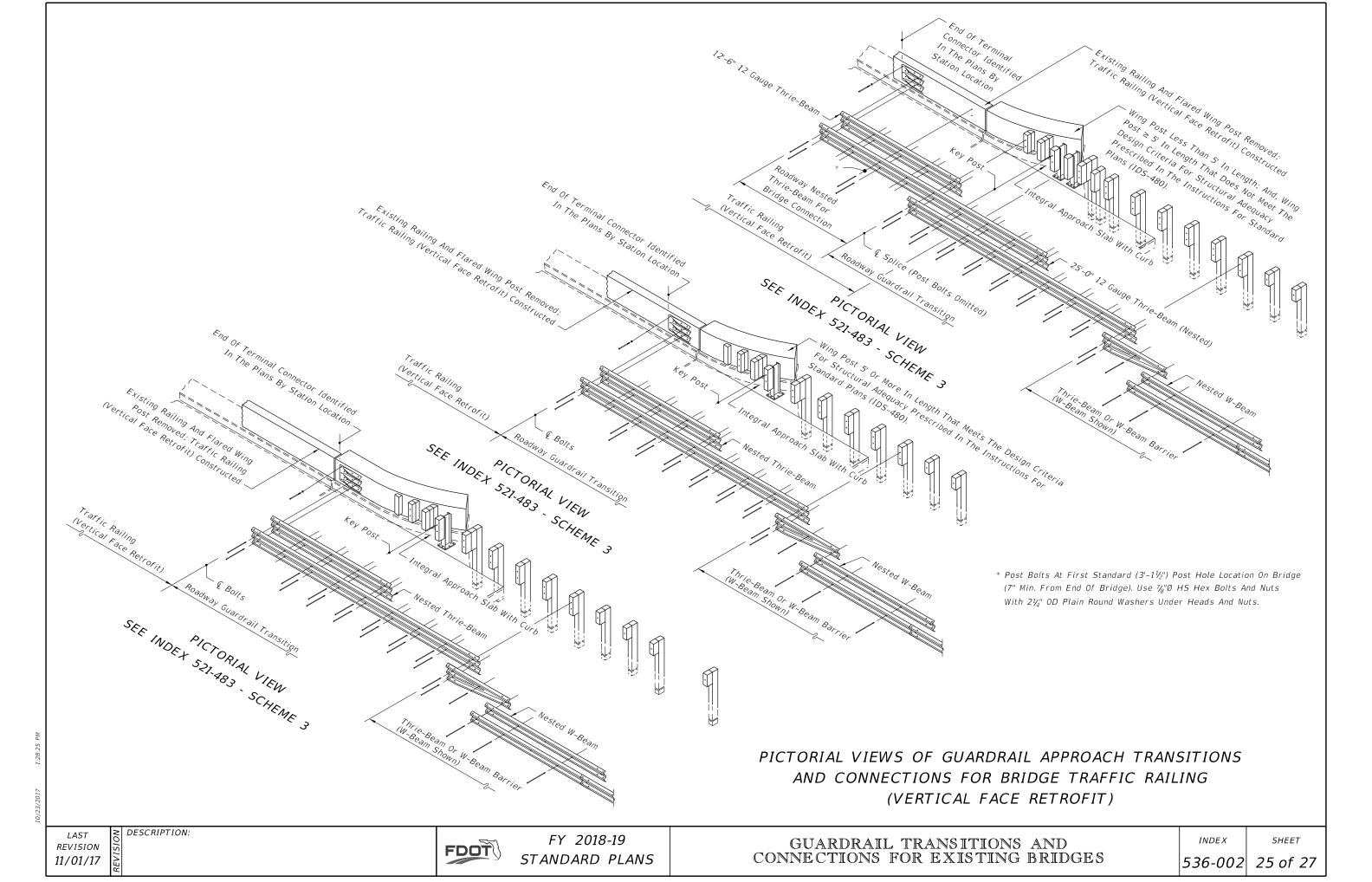
PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 521-482 SHOWN, INDEX 521-405 SIMILAR)

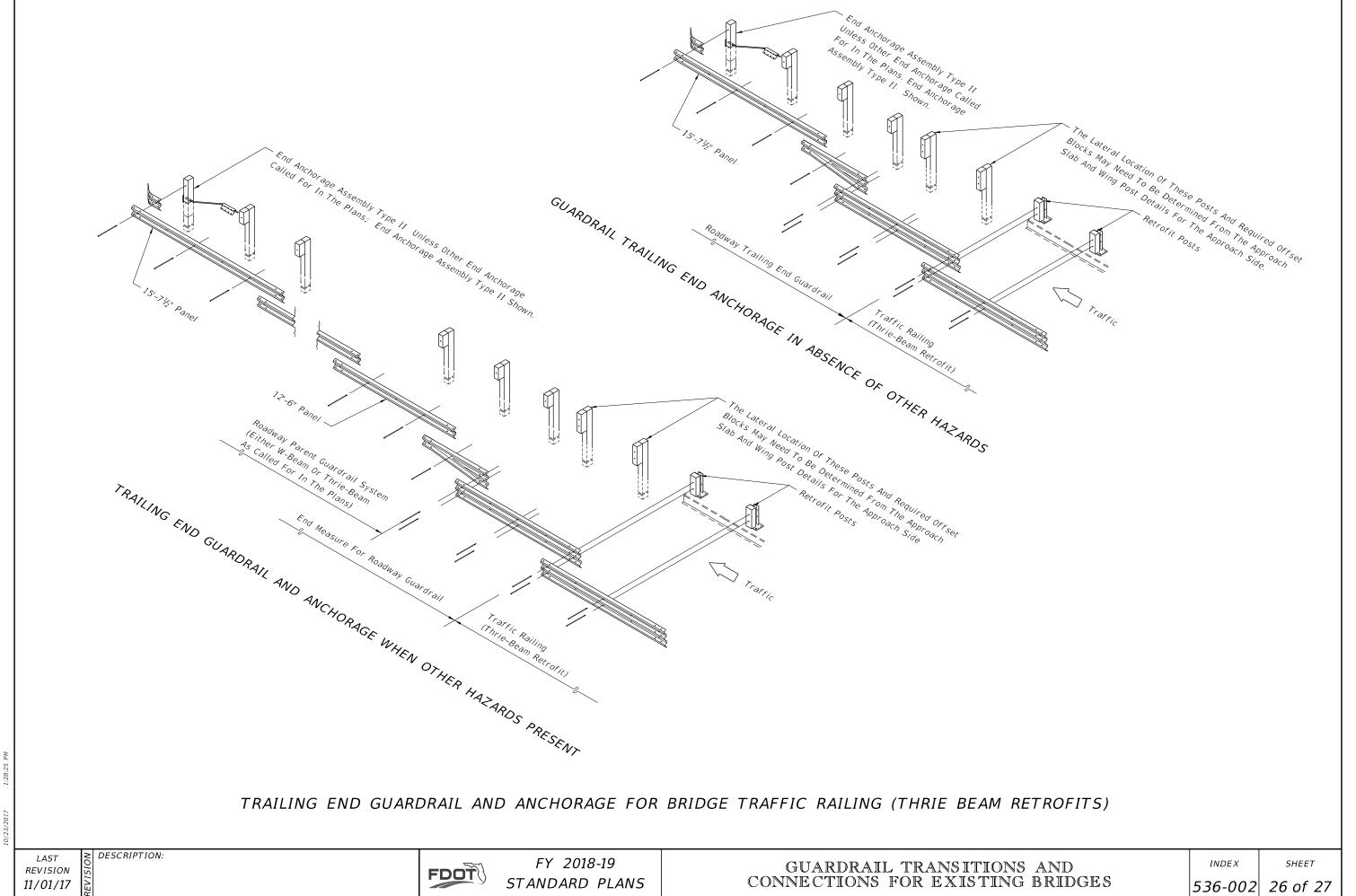
LAST REVISION 11/01/17

DESCRIPTION:

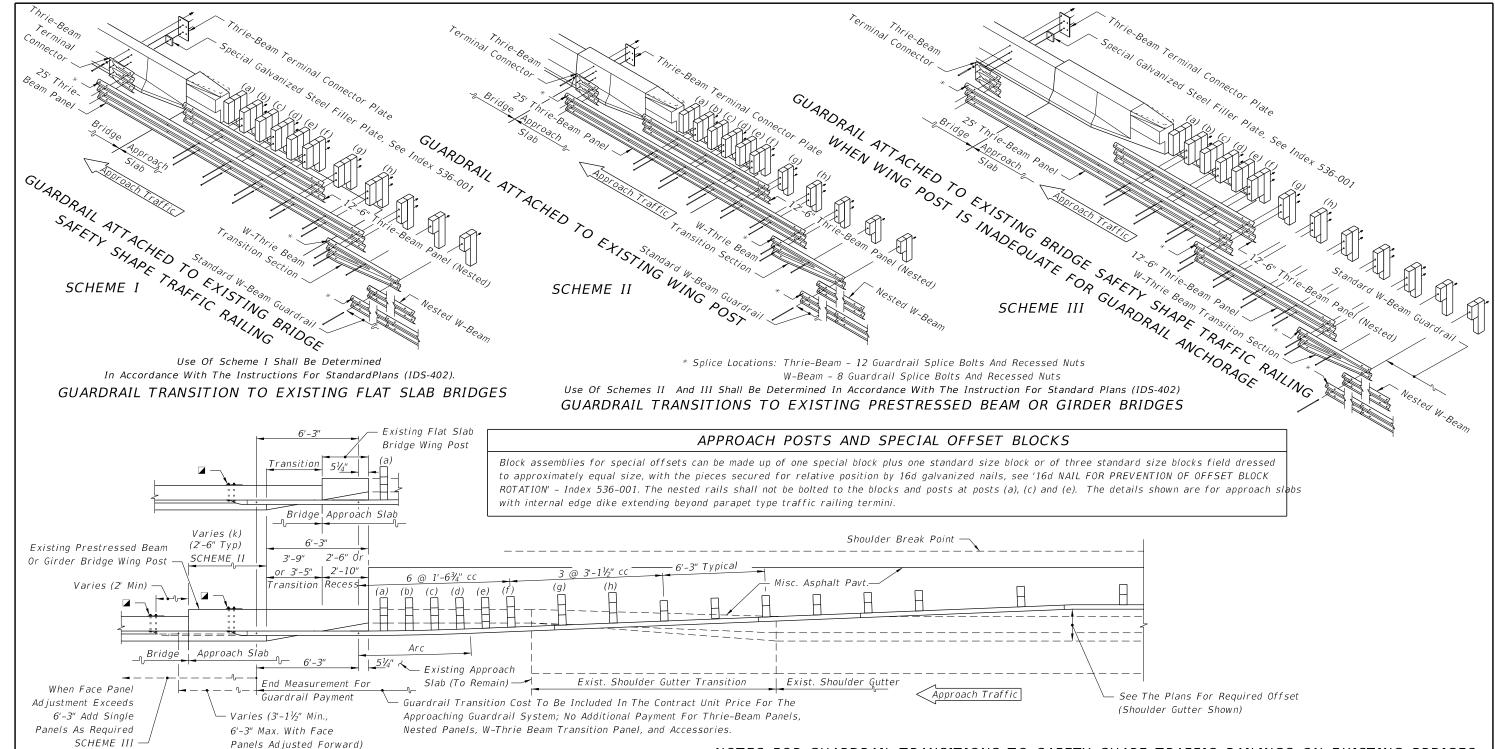
FDOT







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🖬 21"x12"x¾" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And ¾"Ø x 18" Long [15" Long With 3½" Min. Thread Length For Bridge Safety Shape Railing] HS Hex Bolts And Nuts (5 Reqd.) With 21/4" OD Plain Round Washers Under Heads And Nuts. [When Attaching Guardrail To Existing Wing Posts Or Bridge Rails, Care Should Be Exercised To Avoid Damaging Conduits And Their Utilities That May Be Routed Through Wing Posts Or Bridge Rails. When Conduits And Their Utilities Are Encountered, At Least Five 7/8" HS Hex Bolts Shall Be Installed In Any Of The Seven Holes Provided In The Thrie-Beam Terminal Connector.]

SCHEME III

NOTES FOR GUARDRAIL TRANSITIONS TO SAFETY SHAPE TRAFFIC RAILINGS ON EXISTING BRDIGES

- 1. When the guardrail attachment overlays the Bridge Number, Bridge Name or Date on the traffic railing, provide an aluminum sign panel with the obscured information. Attach the sign panel to the face of the traffic railing adjacent to the Thrie-Beam Terminal Connector with 1/4"0 x 1" long concrete screws or expansion anchors at each corner, as approved by the Engineer. The sign panel shall be a minimum $\frac{1}{16}$ " thick and meet the requirements of Specification Section 700 with a white background and 3" tall black letters and sized appropriately to contain the information required. The cost of the sign panel shall be included in the cost of the Guardrail Bridge Anchorage Assembly.
- 2. When retrofitting thrie-beam guardrail to existing wing posts or existing bridge safety shape traffic railing, attachment construction to be paid for under the contract unit price for Guardrail Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate(s) and bolts, nuts and washers.

GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR EXISTING FLAT SLAB, PRESTRESSED BEAM AND GIRDER BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH

REVISION 11/01/17

DESCRIPTION:

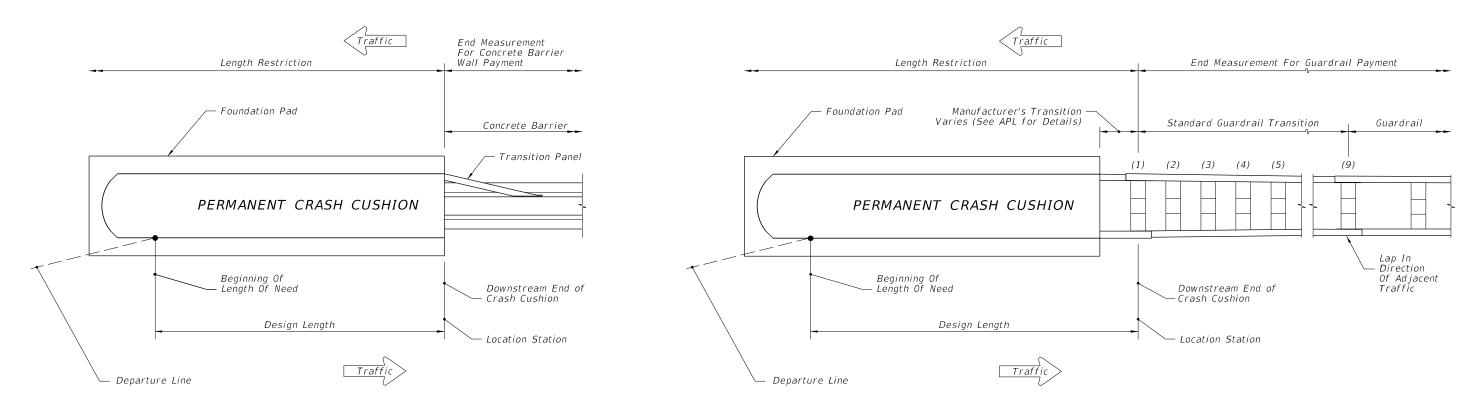
FY 2018-19 STANDARD PLANS

PLAN

GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

SHEET INDEX

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Concrete Barrier Applications							
Design Length (ft.)	Design Speed (mph)	Crash Test Level					
5.75	35						
7.25	40	TL-2					
7.25	45						
10.25	50						
13.25	55	TL-3					
16.00	≥ 60						

GENERAL NOTES

- 1. This Index is applicable for permanent crash cushion installations that shield the ends of Concrete Barrier or Guardrail, only.
- 2. Design Length is based on a given design speed and the shortest Crash Cushion available on the Approved Products List (APL). When a Length Restriction is not applicable (N/A), then the Contractor has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans. When a Length Restriction is applicable, then the Contractor has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans and that are less than or equal to the Length Restriction identified in the plans.
- 3. For High Speed Facilities with a Design Speed greater than 60 mph, use a TL-3 Crash Cushion.
- 4. Assemble and install Crash Cushions according to the limitations noted on the Approved Products List (APL) webpage, the manufacturer's specifications, and the applicable crash cushion drawings posted on the APL.
- 5. When subjected to reverse direction hits, construct Transition Panels from Concrete Barrier to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.
- 6. Galvanize metallic components are to meet the requirements in the Specification, Section 967.
- 7. For Guardrail Applications, construct the Manufacturer's Transition between the Permanent Crash Cushion and the Standard Guardrail Transition; refer to all Standard Guardrail Transition details of this Index.
- 8. For additional information on the End Measurement for Guardrail Payment, refer to the Standard Specifications for Road and Bridge Construction, Section 536.
- 9. Provide delineation in accordance with Specification, Secton 544.
- 10. The EOR shall provide the station of the Length of Need (LON) location in the plans.

Guardrail Applications						
Design Length (ft.)	Design Speed (mph)	Crash Test Level				
8.75	35					
11.50	40	TL-2				
11.50	45					
14.25	50					
20.00	55	TL-3				
22.75	≥ 60					

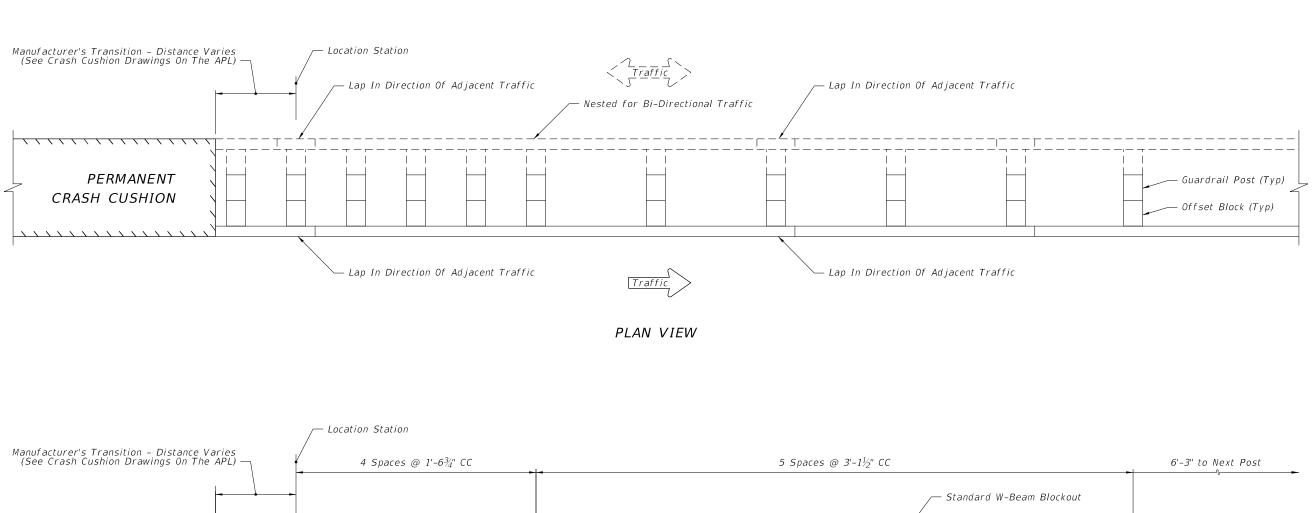
PERMANENT CRASH CUSHION APPLICATIONS

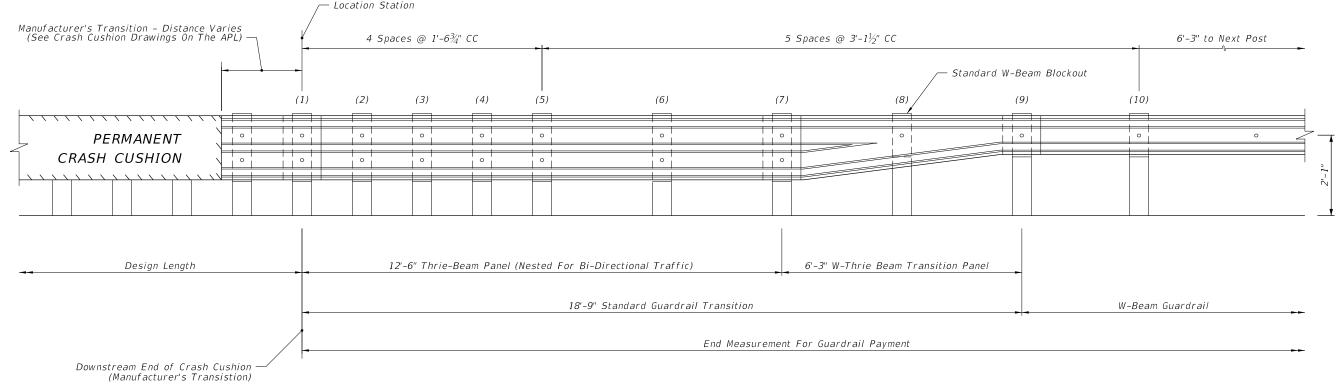
LAST REVISION 11/01/17

DESCRIPTION:

FDOT

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STANDARD PLANS





ELEVATION VIEW

Post Numbers 8, 9 and 10 will have Standard 6"x8"x14" Wooden W-beam Blockouts.

For Additional Information on Standard Guardrail Transitions see Index 536-001.

STANDARD GUARDRAIL TRANSITION

LAST **REVISION** 11/01/17

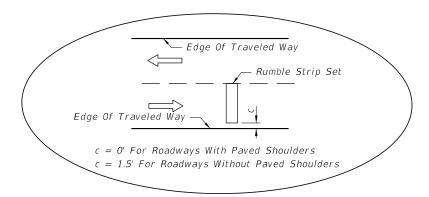
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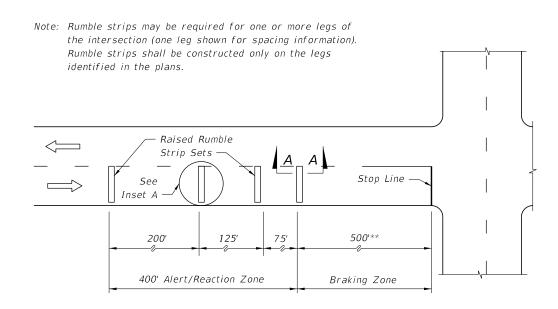
FY 2018-19 STANDARD PLANS

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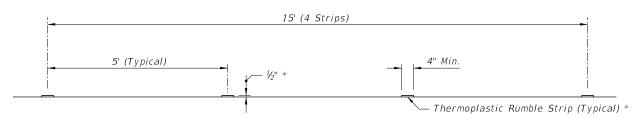


INSET A



** May be decreased in urban areas with low operating speeds.

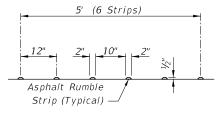
PLAN



* Use multiple applications to achieve desired $\frac{1}{2}$ " thickness

Note: Intersection thermoplastic rumble strip sets shall be white.

THERMOPLASTIC SET



ASPHALT SET

SECTION AA FOR THERMOPLASTIC AND ASPHALT RUMBLE STRIP SETS

NOTE:

Raised rumble strips are to be constructed in accordance with Section 546 of the Specifications.

RAISED RUMBLE STRIPS AT INTERSECTIONS

REVISION 11/01/17

DESCRIPTION:

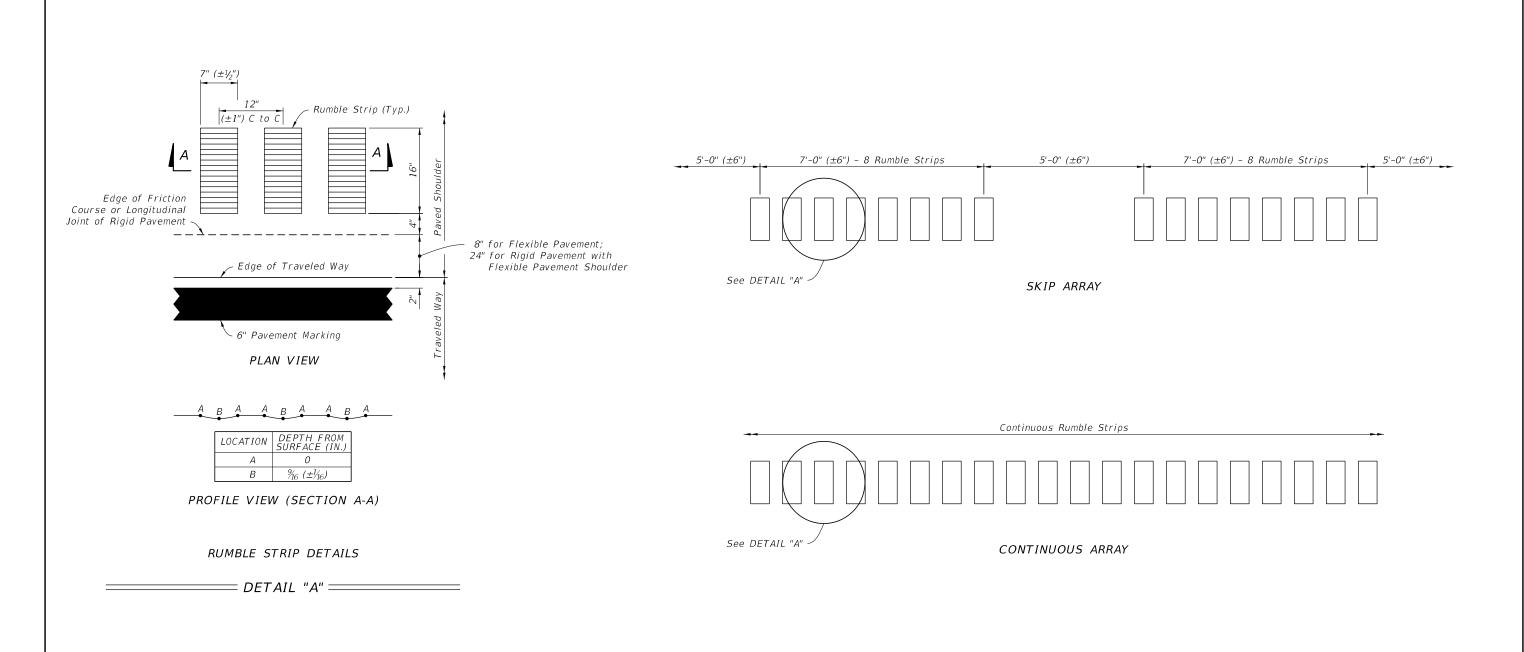


FY 2018-19 STANDARD PLANS

RAISED RUMBLE STRIPS

INDEX

SHEET 1 of 1



GROUND-IN RUMBLE STRIPS FOR LIMITED ACCESS ROADWAYS

GENERAL NOTES

- 1. For Limited Access roadways, when friction course extends more than 8" beyond the edge of the traveled way, blade off the extended friction course to the 8" line prior to rumble strip grinding.
- 2. Use the continuous array on both inside and outside shoulders in advance of bridge ends for a distance of 1,000 feet or back to the gore recovery area for mainline interchange bridges. Use the skip array for all other locations.
- 3. Exclude rumble strips at the following locations:
- A. At mainline tolling areas, terminate rumble strips at the end of the mainline normal section.
- B. At All Electronic Tolling (AET) facilities, terminate rumble strips within 50 feet of the centerline of the overhead gantry.
- C. On outside shoulders of entrance ramp terminals, terminate rumble strips at the point of the physical gore and resume at the end of the acceleration lane taper.
- D. On outside shoulders of exit ramp terminals, terminate rumble strips at the start of the deceleration lane taper and resume at the point of the physical gore.
- E. On approaches to bridges, terminate rumble strips at the approach slab joint.
- F. On either side of median crossover openings, terminate rumble strips within 400 feet.

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2018-19 STANDARD PLANS

SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

1. See Specification Section 548 for material requirements.

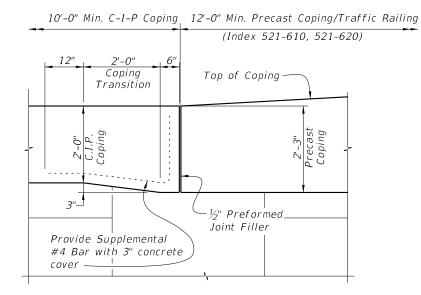
CONSTRUCTION:

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. If required, locate manholes and drop inlets as shown on wall elevations.
- 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.
- 9. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 10. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 11. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 12. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- 13. The top of the leveling pad or footing will be 2'-0" minimum below final ground line.
- 14. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 15. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 16. Work this Index with Index 521-600 thru 521-650.

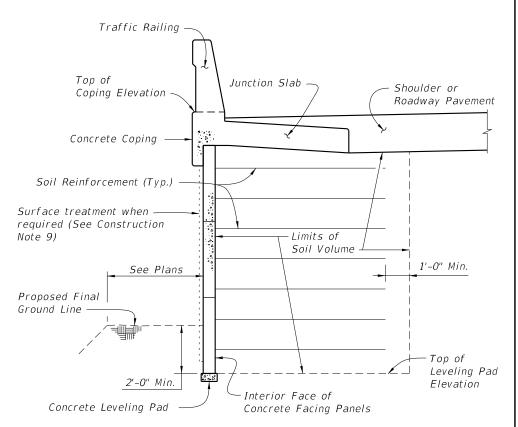
SHOP DRAWING REQUIREMENTS:

DESCRIPTION:

See Specification Section 548 for shop drawing requirements.



ELEVATION VIEW OF COPING HEIGHT TRANSITION (Railing Not Shown For Clarity)



TYPICAL MSE RETAINING WALL SECTION WITH A TRAFFIC RAILING (Showing Limits of the Reinforced Soil Volume)

FDOT MSE RETAINING WALL CLASSIFICATION TABLE										
Durability Requirements					0the	r Allo	wable	FDOT	Wall	Types
Applicable	Concrete	Concrete	Pozzolan	Soil						
FDOT Wall	Cover	Class	Additions?	Reinforcement	2A	2B	2C	2D	2E	2F
Type *	(in.)	for Panels	**	Туре						
Type 2A	2	II	No	Metal		/	/	1	/	1
Type 2B	2	IV	No	Metal			1	1	1	/
Type 2C	3	IV	No	Metal				1	1	/
Type 2D	3	IV	Yes	Metal					1	/
Type 2E	3	IV	No	Plastic						/
Type 2F	3	IV	Yes	Plastic						

- * See Data Table in Contract Plans.
- ** Silica fume, metakaolin or ultrafine fly ash.

GENERAL NOTES AND DETAILS

REVISION 11/01/17

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FY 2018-19 STANDARD PLANS

DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and FDOT Structures Design Guidelines Section 3.13.2.
- 2. It is the responsibility of the Engineer to determine that the factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer.
- 4. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.

SOIL PARAMETERS:

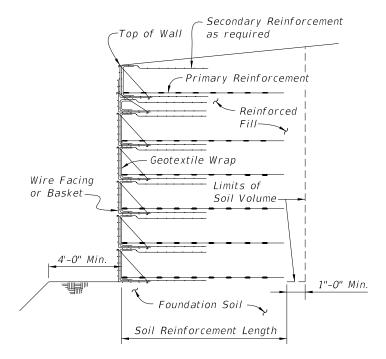
1. See wall control drawings for soil characteristics of foundation material to be used in the design of the wall system. The Contractor must provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site. Provide the values of unit weight, cohesion and internal friction angle in the Shop Drawings.

MATERIALS:

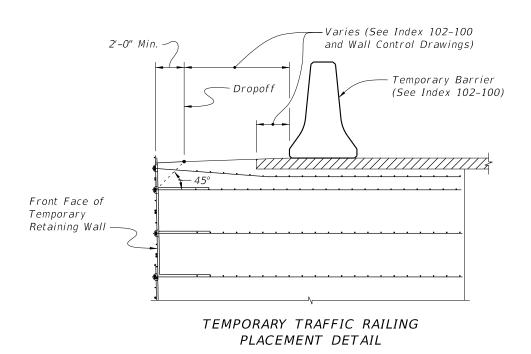
- 1. Provide soil reinforcement in accordance with Specification Section 548.
- 2. For additional material notes, see Wall Company General Notes.

CONSTRUCTION:

- 1. Walls must be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. Refer to Plan and Elevation sheets of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 4. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor must notify the Engineer to determine what course of action should be taken.
- 5. The Contractor is responsible for gradually deflecting upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.



TYPICAL RETAINING WALL SECTION (Showing Limits of the Reinforced Soil Volume)



GENERAL NOTES AND DETAILS

LAST REVISION 11/01/17

DESCRIPTION:

FDOT

SHEET

GENERAL NOTES

- 1. This fence to be provided generally in rural areas. For supplemental information see Specifications 550.
- 2. Fabric shall be woven wire, either galvanized steel, meeting the requirements of ASTM A116, No. 9 Grade 60, Design Number 1047-6-9, with Class 3 zinc coating; No. 12 ½ Grade 175, Design Number 1047-6-12 ½, with a 10 ½ gage top and bottom wire and with Class 3 zinc coating; or aluminum coated steel, meeting the requirements of ASTM A584, No. 9 Farm, Design Number 1047-6-9, with a minimum coating weight of 0.40 oz./ft.². For additional information see payment note below.
- 3. Fence shall be installed with wire side to private property except on horizontal curves greater than 3° the fence shall be installed so as to pull against all posts.
- 4. Posts may be either timber, steel, recycled plastic or concrete. Unless a specific post material is called for in the plans, the Contractor may elect to use either a single material or a combination of timber, steel, recycled plastic or concrete materials, but must comply with the electrical grounding requirements in Specifications 550. Line posts of one material may be used with corner, pull and end post assemblies of a different material. Line posts of only one optional material and pull post assemblies of only one optional material will be permitted between corner and end post assemblies. Within individual corner and end post assemblies only one optional material will be permitted.
- 5. Timber posts shall meet the material requirements of Specification 954. Timber line posts are to be minimum 4" diameter. Timber corner, pull, approach and end posts are to be a minimum 5" diameter. Timber braces are to be minimum 4" diameter.
 - (A) Staples for line posts to be $1\frac{1}{4}$ " minimum length; for approach, corner and pull posts $1\frac{1}{2}$ " minimum length. At approach, corner and pull posts, staple every line wire. At line posts, staple every line wire in top half and alternate line wires in bottom half. Staples shall be driven diagonally across the line wire with the points in separate grains.
 - (B) Connections between timber posts and braces to be provided by dowels as shown in fastener details.
 - (C) Wire to be wrapped and tied, as shown in the splice details, at the following locations:
 - (a) All end posts, (b) Corner post, including the assemblies at vertical breaks of 15° or more and (c) Pull posts where the wire is not spliced and pulled through the assembly; see General Note 18.
 - (e) Tan posts where the wife is not spriced and parted through the assembly, see deficial note is
- 6. Steel posts and braces shall be standard steel posts, galvanized at the rate of 2 oz./ft.², together with necessary hardware and wire clamps and meeting the following requirements:
 - (A) Line posts: 8' long; 1.33 lbs./ft.; roll formed studding; anchor plate attached, ASTM A702 (18 in.2).
 - (B) Approach posts: 2½"x2½"x½" angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - (C) Pull, end and corner posts: $2\frac{1}{2}"x2\frac{1}{2}"x\frac{2}{4}"$ angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - (D) Braces: $2"x2"x\frac{1}{4}"$ angles with necessary hardware and fabricated for attaching to post.
 - (E) The pull, corner, approach and end posts are to be set in concrete as per detail. (Also see General Note No. 15)
- 7. Recycled plastic posts shall meet the following material requirements: Line posts shall have a minimum section of 4" round or 4" square. Plastic posts shall not be used as corner, pull, end or approach posts unless such use is specifically detailed in the plans. The straightness of the post shall comply with Specification 954 for timber post. The flexural strength shall meet the requirements of the latest edition of the Southern Pine Inspection Bureau's Standard Grading Rules for Southern Pine Lumber for No. 25R Stress Rated Grade Timber. Plastic posts can be set by either digging and tamped backfill or by driving into full depth preformed holes ¼" to ½" smaller than cross section of post.
 - Staples for fabric and barbed wire connection to plastic line posts shall be the same size, count and location as that for timber posts.
- 8. The Contractor, at his option, may use any suitable precast or prestressed concrete posts; however, approval by the Engineer, of posts not shown on this index, will be required prior to construction of the fence. Precast posts shall be Class I concrete. Prestressed posts shall be Class III concrete. Lengths of concrete post to be as indicated for timber posts.
- 9. Aluminum post, braces and accessory framing hardware shall not be used unless the plans specifically detail their application or the Engineer specifically approves their incorporation in fence construction or repair. Aluminum framed gates are permitted as described in General Note 19.

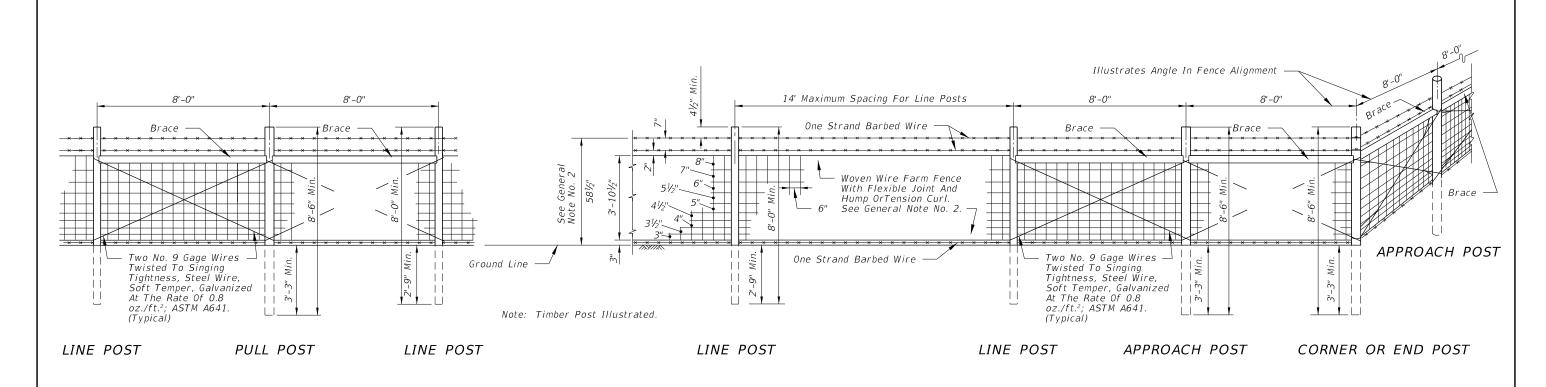
- 10. The woven wire shall be attached to steel and concrete posts by a minimum of five tie wires. The single wire ties shall be applied to the top, bottom and three intermittent line wires. The ends of each tie wire shall have a minimum of two tight turns around the line wire. Tie wires shall be steel wire not less than 0.120" diameter, zinc coating Class 3, soft temper, in accordance with ASTM A641.
- 11. Steel Barbed Wire can be either of the following types:
 - Type I: This type shall conform to the requirements of ASTM A121, with two strands of 12½ gage wire; four-point barbs, wire size 14 gage, twisted around both line wires; and, Class 3 coating, Design No. 12-4-5-14R.
 - Type IIA: This type same as Type I except the two strand wires are twisted in alternating directions between consecutive barbs.
 - Type IIB: This type shall conform to the requirements of ASTM A121 with two strands of 15 $\frac{1}{2}$ gage high tensile wire; four-point barbs, wire size 16 $\frac{1}{2}$ gage twisted around both line wires; and Class 3 coating, Design No. 15-4-5-16R.

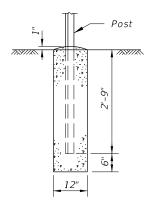
Aluminum Barbed Wire shall be fabricated of two strands of 0.110-inch wire with 0.08-inch diameter four-point barbs spaced at approximately $5\frac{1}{2}$ ", and at a maximum spacing of 6". The wire for the strands and for the barbs shall be of ASTM B211M Alloy 5052-H38 or equal.

- 12. The woven wire shall be stretched only until one-half the tension curl has been pulled out of the line wires.
- 13. Posts to be set by driving or digging. If by digging, the posts shall be set at the center of the hole and the soil tamped securely on all sides.
- 14. Longer posts than those indicated above may be required by the plans or for deeper installations.
- 15. Concrete bases for angular steel posts (pull, corner, end and approach) shall be Class NS as specified in Section 347. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 16. Pull post assemblies shall be installed at approximately 330' centers except that this maximum interval may be reduced by the Engineer on curves where the radius is less than 3°.
- 17. Corner post assemblies are to be installed at all horizontal and vertical breaks in fence of 15° or more.
- 18. A maximum length of 1320' of wire may be installed as a unit. For pulls through a pull post assembly the fabric shall be spliced by crimping sleeves only. Pulls through a corner post assembly will not be permitted.
- 19. Unless otherwise called for in the plans gates shall be commercially available metal swing gates assembled and installed in accordance with the manufacturer's specifications as approved by the Engineer. Chain link swing gates in accordance with Index 550-002 may be substituted for metal swing gates as approved by the Engineer. Gate size is full opening width whether single leaf or double leaves. Payment for gates shall include the gate, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 20. For construction purposes, assemblies are defined as follows: End post assemblies shall consist of: one end post, one approach post, two braces, four diagonal tension wires and all necessary fittings and hardware. Pull post assemblies shall consist of: one pull post, two braces, four diagonal tension wires and necessary fittings and hardware. Corner post assemblies shall consist of: one corner post, two approach posts, four braces, eight diagonal tension wires and all necessary fittings and hardware.
- 21. All posts, braces, tension wires, fabric, tie wires, Class NS concrete, and all miscellaneous fittings and hardware to be included in the cost for Fencing, LF. Fencing shall be inclusive of the lengths of pull, end and corner post assemblies, but exclusive of gate widths.

DESCRIPTION:

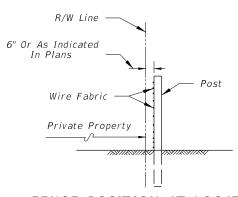
1 of 3





(Pull, Corner, End And Approach Posts)

CONCRETE BASE FOR ANGULAR STEEL POST



FENCE POSITION AT LOCATIONS WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)

DESIGN NOTE

This index details fencing that is constructed with farm fabric $46\frac{1}{2}$ " (47" nominal) in height and with specific ground clearance and specific barbed wire spacings. For fencing of different height or installation details, the fence shall be fully detailed in the Contract plans.

REVISION 11/01/17

DESCRIPTION:

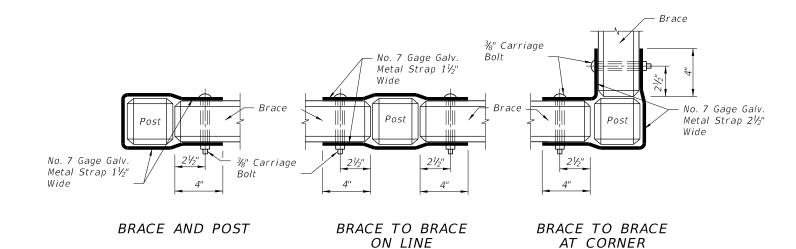
FDOT

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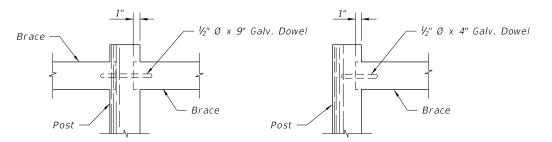
FENCE TYPE A

INDEX 550-001

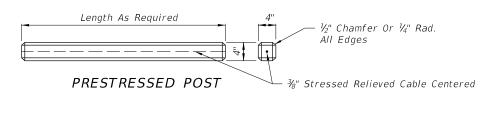
SHEET

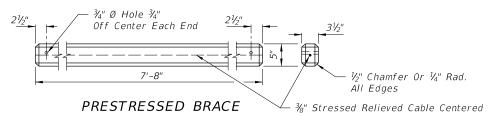


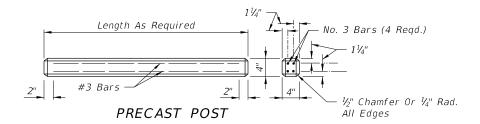
FASTENER FOR CONCRETE POST AND BRACES

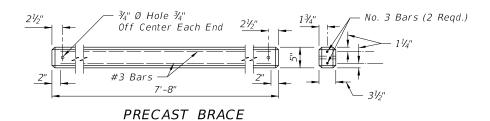


FASTENER FOR TIMBER POST AND BRACE

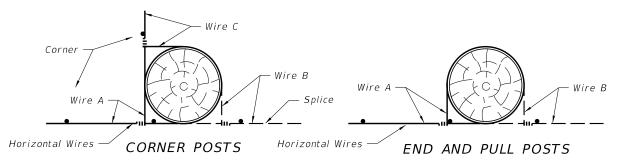








ALTERNATE CONCRETE POSTS AND BRACES



Each horizontal wire to be wrapped around corner, end and pull posts and tied to same wire. See General Notes 5 and 17. Timber post illustrated. These methods also apply to steel and concrete post illustrations.

SPLICES

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LAST REVISION 11/01/17

DESCRIPTION:

FDOT

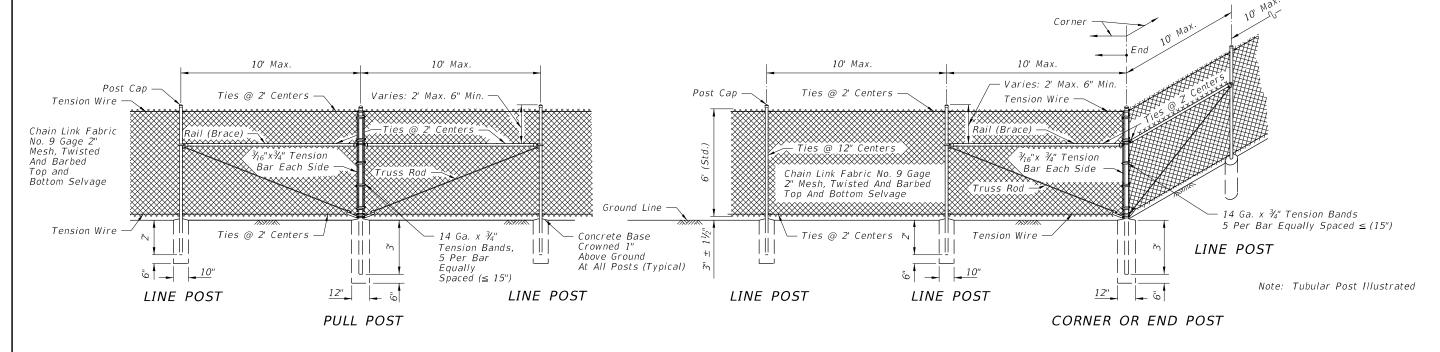
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FENCE TYPE A

INDEX

SHEET

PE A 550-



GENERAL NOTES

- 1. This fence to be used generally in urban areas.
- 2. For supplemental information refer to Section 550 of FDOT Standard Specifications.
- 3. Chain link fabric, post, truss rods, tension wires, tie wires, stretcher bars, gates and all miscellaneous fittings and hardware shall meet the requirements of AASHTO and ASTM signify current reference.
- 4. Fence Component Options:
 - A. Line post options:
 - (1) Galvanized steel pipe, Schedule 40- $1\frac{1}{2}$ " nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table 2 (Grade A or B), ASTM F1083, and AASHTO M111.
 - (2) Aluminum coated steel pipe: ASTM A53, Table 2 (Grade A or B): Schedule 40- 11/2" nominal dia., 1.90" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
 - (3) Aluminum alloy pipe- 2" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
 - (4) Steel H-Beam- 11/8" x 15/8": Zinc Galv. 1.8 oz./ft.: AASHTO M111 and Detail.
 - (5) Aluminum alloy H-Beam- 17/8" X 15/8" Detail.
 - (6) Steel C- 1%"X 15%": Galv.: 1.8 oz/ft. zinc: AASHTO M111; OR , 0.9 oz./ft². zinc-5% aluminummischmetal: ASTM F1043 and Detail.
 - (7) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 2" OD, 11/2" NPS, 1.900" dec. equiv., 0.120" min. wall thick, and min. wt. 2.28 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15μg/in². min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.

 - B. Corner, end, and pull post options:

 (1) Galvanized steel pipe, Schedule 40- 2" nominal dia. zinc galvanized at the rate of 1.8 oz./ft².:

 ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.

 (2) Aluminum coated steel pipe: ASTM A53 steel, X 2 Tables: Schedule 40; 2" nominal dia.,

 2.375" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.

 - (3) Aluminum alloy pipe- $2\frac{1}{2}$ " nominal dia.: ASTM B241 or B221, Alloy 6063,T6.
 - (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry $2\frac{1}{2}$ " OD, 2" NPS, 2.375" dec. equiv., 0.130"min. wall thick. and min. wt. 3.117 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of $15\mu g/in^2$. min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.

C. Rail options:

- (1) Galvanized steel pipe, Schedule 40- $1\frac{1}{4}$ " nominal dia. zinc galvanized at the rate of 1.8 oz./ft².: ASTM A53 Table X 2, ASTM F1083, and AASHTO M111.
- (2) Aluminum coated steel pipe; ASTM A53 steel, X 2 Tables Schedule 40; 11/4" nominal dia., 1.660" OD; coated at the rate 0.40 oz./ft.: AASHTO M111.
- (3) Aluminum alloy pipe- 11/4" nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
- (4) Resistance welded steel pipe; 50,000 psi min. yeild strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 15/8" OD, 11/4" NPS, 1.660" dec. equiv., 0.111" min. wall thick. and min. wt. 1.836 lb./ft.; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of $15\mu g/in^2$. min. and the polymer film topcoat shall have a thickness of 0.003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043
- D. Chain link fabric options (2" mesh with twisted and barbed selvage top and bottom for all options except as described in Note No. 10):
- (1) AASHTO M181 Type I Zinc Coated Steel, No. 9 gage (coated wire diameter), coated at the rate of 1.8 oz/ft². (M181 Class D 2.0 oz./ft². modified to 1.8 oz./ft².).
- (2) AASHTO M181 Type II -Aluminum Coated Steel, No. 9 gage (coated wire diameter), coated
- (3) AASHTO M181 Type IV- Polyvinyl Chloride (PVC) Coated Steel, No. 9 guage (coated core wire diameter), core wire-zinc coated steel. PVC coating: M181 Class A (either extruded or extruded and bonded) or Class B (bonded). See table right. Unless the plans call for M181 standard colors medium green, dark green or black the coating color shall be soft gray matching that of No. 36622 of Federal Standard 595a.

E. Tension wire options:

- (1) Steel wire No. 7 gage zinc galvanized at the rate of 1.2 oz./ft².: AASHTO M181.
- (2) Aluminum alloy wire with a diameter of 0.1875" or larger conforming to the requirements of ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
- (3) Aluminum coated steel wire No.7 gage coated at the rate of 0.040 oz./ft².: AASHTO M181.

F. Tie wire and hog ring options:

- (1) Steel wire No.9 gage zinc galvanized at the rate of 1.2 oz./ft².
- (2) Aluminum alloy wire with a diameter of 0.1443" or larger conforming to the requirements of
- ASTM B211, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192. (3) Aluminum coated steel wire No. 7 gage coated at the rate of 0.040 oz./ft².

DESCRIPTION:

(a) Only one fabric optional material will be permitted between corner and/or end post assemblies.

- (b) Only one line post optional material will be permitted between corner and/or end post assemblies.
 (c) Pull post assemblies shall be optional materials identical to either the linepost optional material or the corner and end post assembly optional material; but, pull post assemblies shall be the same optional material between any set of corner and/or end post assemblies.
- 6. Concrete for bases shall be Class NS concrete as specified in Section 347 of the Standard Specifications or a packaged, dry material meeting the requirements of a concrete under ASTM C-387. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 7. Line post shall be 8'-6" long (Standard). Line post are to be set in concrete as described above or by the following methods:
 - (a) In accordance with special details and/or as specifically described in the contract plans and specifications.
 - (b) In accordance with ASTM F567 Subsections 5.4 through 5.10 as approved by the Engineer. Line post installed in accordance with Section 5.8 shall be 9'-6" long.
 - (c) Post mounted on concrete structure or solid rock shall be mounted in accordance with the base plate detail "Fence Mounting On Concrete Endwalls And Retaining Wall", Sheet 3; or, by embedment in accordance with ASTM F567 Subsection 5.5.

End, pull and corner post assemblies shall be in concrete as detailed above for all soil conditions other than solid rock. Post within assemblies that are located on concrete structures or solid rock shall be set by base plate or by embedment as prescribed under (b) above for line post.

Line and assembly posts for 6' fence which must be lengthened due to a variation in the normal ground clearance, shall be set an additional 3" in depth for each 1' of of additional ground clearance.

- 8. Pull post shall be used at breaks in vertical grades of 15° or more, or at approximately 350' centers except that this maximum interval may be reduced by the Engineer on curves where the curve is greater than 3°.
- 9. Corner post are to be installed at all horizontal breaks in fence at 15° or more and as required at vertical breaks over 15° as determined by the Engineer.
- 10. When fence has an installed top of fabric height less than 6' knuckled top and bottom selvages shall be used unless the plans specifically identify locations for twisted selvage fabrics.
- 11. Unless sliding gates or special gates are called for in the plans, all gates shall be chain link swing gates meeting the material requirements described and as approved by the Engineer. Payment shall include the gates, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, EA.
- 12. For construction purposes corner post assemblies shall consist of one corner post, two braces, two truss rods, and all necessary fittings and hardware as detailed. End post assemblies shall consist of one end post, one brace, one truss rod and all necessary fittings and hardware as detailed.
- 13. In areas where there are physical constraints outside the right-of-way which restricts the fence construction, the fabric may be installed on the inside of the posts..

TYPE IV VINYL COATED FABRIC									
	AASHTO M181 Table 4 Redefined As Follows								
					PVC Thickness Range				
Specif Of Me Core \	ied Dia tallic Co Nire	meter pated		imum Weight Zinc Coating (Extruded Or Extruded And Bonded Coating)		Or Extruded	M181 Class B (Bonded Coating)		
in.	mm	gage	oz./ft².	g/m²	in.	mm	in.	mm	
0.148	3.77	9	0.30	92	0.015 to 0.025	0.38 to 0.64	0.006 to 0.010	0.15 to 0.25	

DESIGN NOTE

This index details fencing that is constructed with chain link fabric 6' (nominal) in height and with specific ground clearance.

For fencing of different height or installation details, the fence shall be fully detailed in the Contract plans.

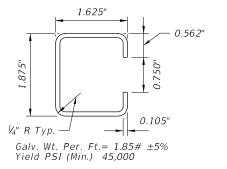
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LAST REVISION 11/01/17

DESCRIPTION:

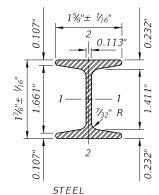




STANDARD WALL

THINWALL

OPTIONAL "C" LINE POST



Area (Sq. In.)
Weight (Lb./Ft.)
Surface Area (SF/Ft.)
Tensile Strength (psi Min.)
Yielding Point (psi Min.)

Moment Of Inertia Section Modulus Rad. Of Gyration

2.72 ± 5% (Galv.) 80,000

Ax	es	Ax	es
1 – 1	2-2	1 – 1	2-2
0.428	0.101	0.428	0.101
0.456	0.124	0.456	0.124

ALUMINUM

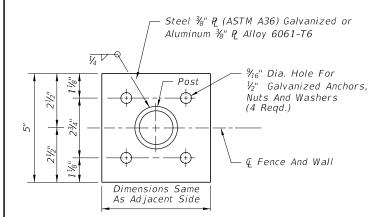
 $0.91 \pm 5\%$ 0.776

0.779 0.373

30,000

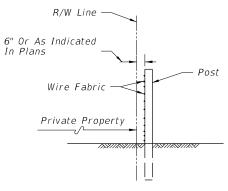
OPTIONAL 17/8" x 15/8" H-BEAM LINE POST

0.779 0.373



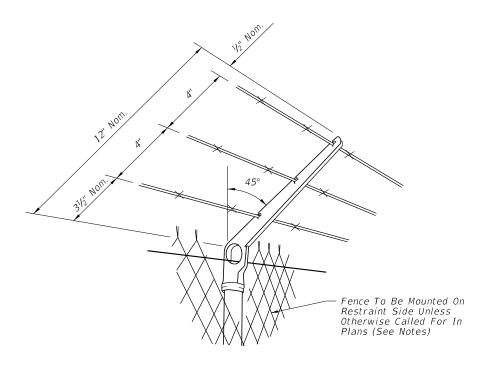
TOP VIEW FOUR ANCHOR PLATE OPTION

DESCRIPTION:



FENCE POSITION AT LOCATIONS WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)



NOTES

Attachments to be used only when called for in the plans.
Attachments to extend in direction of restraint. Unless otherwise called for in plans, direction of restraint will be as follows:

(a.) Outward on limited access right of way line.

(b.) Outward on controlled access right of way line.

(c.) Outward from utilities and hazardous facilities located within highway right of way.

- within highway right of way.
- (d.) Outward from lateral ditches, outfalls, retention basins, canals, borrow areas and similar support facilities.
- (e.) Inward on pedestrian ways.

The cap-arm shall be designed to provide a drive fit over the top of posts and to exclude moisture in posts with tubular sections.

BARB WIRE ATTACHMENT

BASE PLATE AND ANCHOR NOTES:

- 1. Base plate identical for line, pull, end and corner posts and shall be considered an integral part of the respective posts for basis of payment.
- 2. Post to be plumbed by grout shim under base plate.
- 3. Anchors (Galvanized Steel):
 - 12" Cast In Place, 10½" Embedment:
 - Headed Bolts, U-Bolts or Cluster Plates. Adhesive Anchors, 6" Min. Embedment.*
 - *Adhesive anchors shall be headless anchor bolts set in drilled holes with an Adhesive Material System in accordance with Specification Sections 416 and 937; drilled holes shall be 1/8" larger in diameter than the anchor bolt.

Expansion Bolts Not Permitted.

TOP VIEW TWO ANCHOR PLATE OPTION

7/8" Dia. Hole For 3/4"Anchors,

Nuts And Washers (2 Regd.)

FENCE MOUNTING ON CONCRETE ENDWALL AND RETAINING WALLS

└─ Steel ½" ॡ (ASTM A36) Galvanized or

Aluminum 1/2" P2 Alloy 6061-T6

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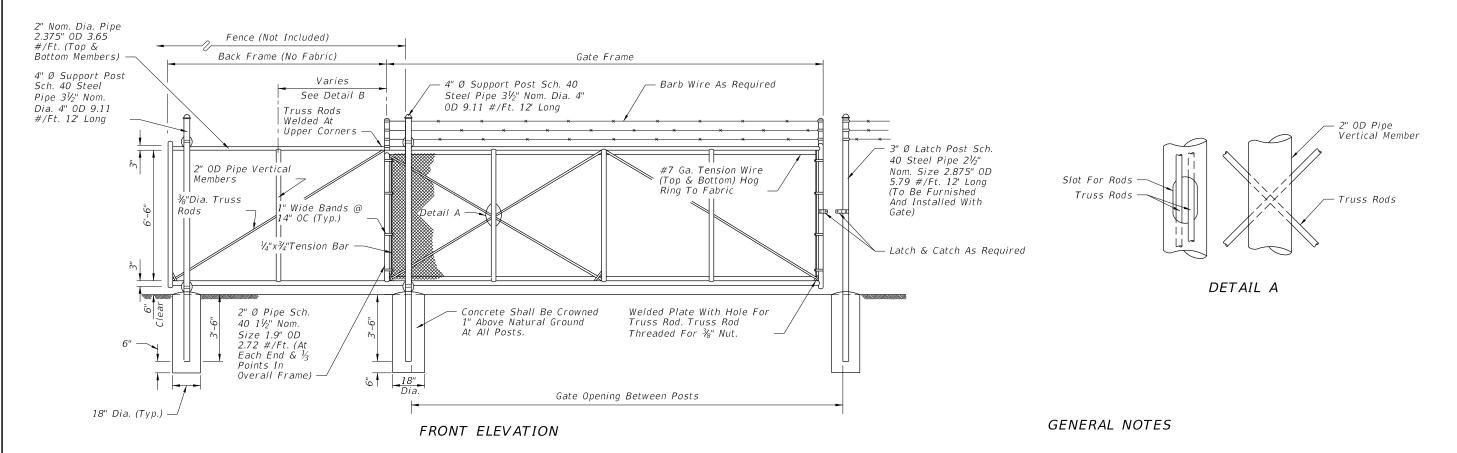
FENCE TYPE B

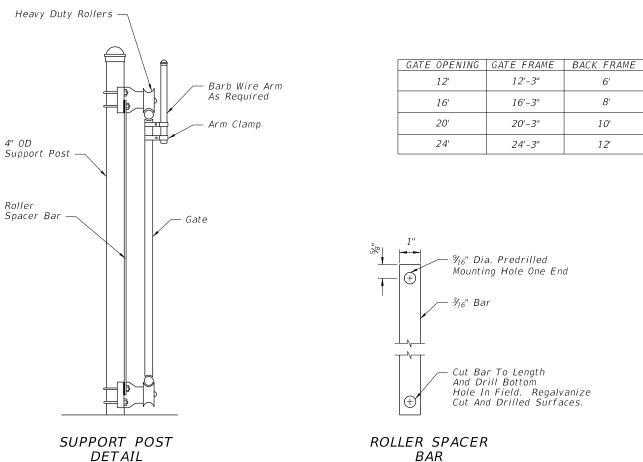
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SHEET

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11/4"



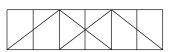


1. Extruded, rolled or formed components that provide equal strength and stability may be used in lieu of the pipe components shown; and, internal rollers may be used in lieu of the external roller units shown.

Gate components shall meet or exceed the protective coatings specified on Index 550-002.

- 2. Steel gate frame shall be fabricated prior to galvanizing, except that truss rods may be fabricated following frame galvanizing provided surfaces damaged during welding are galvanized in accordance with Section 24 of AASHTO M36; or, fabricated from pipe components with protective coating meeting the requirements of Index 550-002 that are tolerant of welding (low burn back), and a protective coating applied to the weld and damaged pipe surfaces that is equivalent to the protective coating of the fabricated pipe
- 3. All fabric shall be knuckled top and bottom selvages.
- 4. Concrete for bases shall be either Class NS concrete as specified in Section 347 of the Standard Specifications or a packaged, dry material meeting the requirements of a concrete under ASTM C-387. Materials for Class NS concrete may be proportioned by volume and/or by weight.
- 5. Cost of all gate components shall be included in the contract unit price for Sliding Fence Gate (Cantilever), EA.





TYPICAL FRAME - 12', 16' & 20' Opening

DETAIL B

REVISION 11/01/17

DESCRIPTION:

FDOT

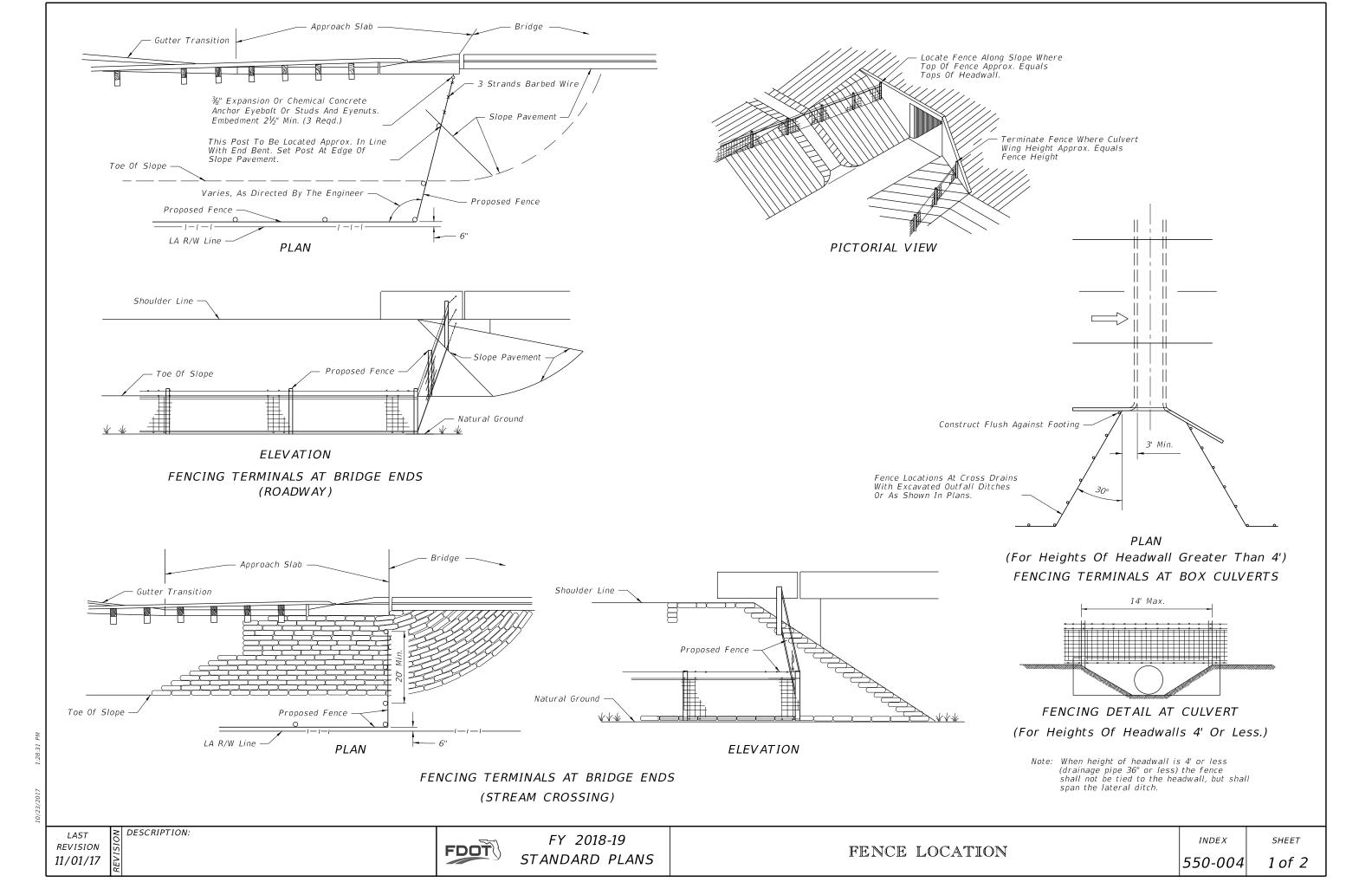
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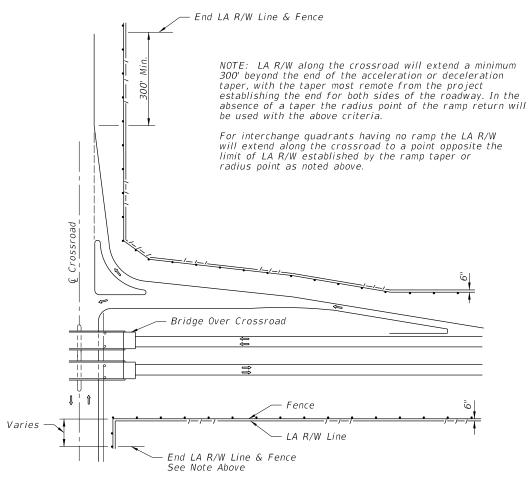
CANTILEVER SLIDE GATE TYPE B FENCE

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SHEET

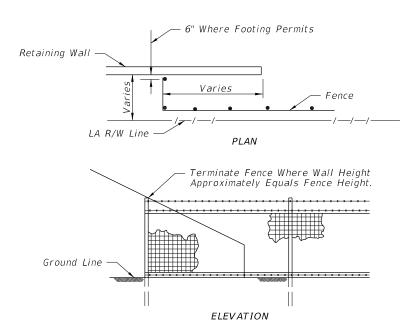
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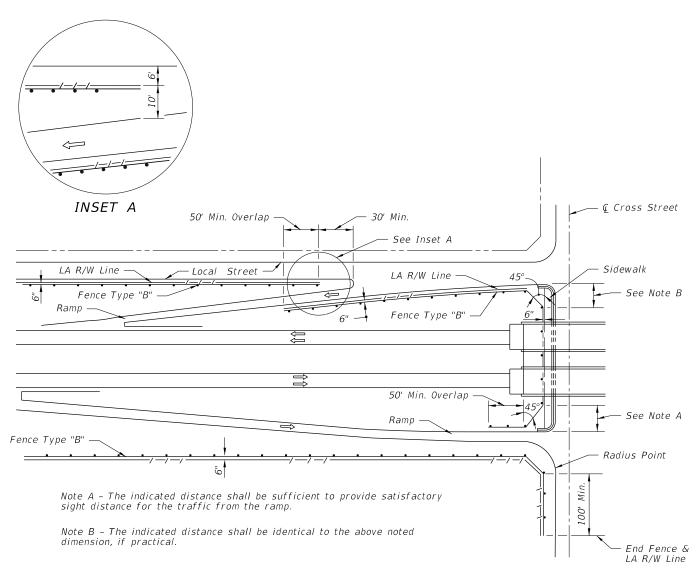


APPLIES TO BRIDGE OVER CROSSROAD AND CROSSROAD OVER FREEWAY (BRIDGE OVER CROSSROAD SHOWN)

FENCING TERMINALS AT RURAL INTERCHANGES

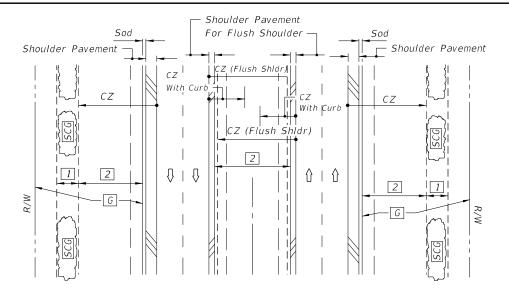


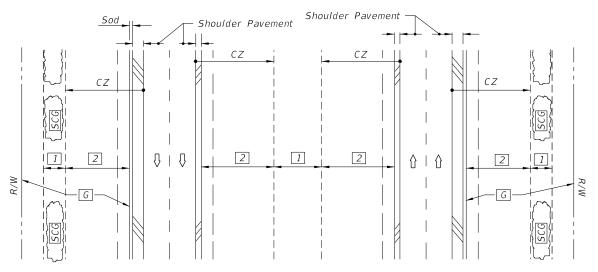
FENCING TERMINALS AT RETAINING WALLS



FENCING TERMINALS AT URBAN INTERCHANGES

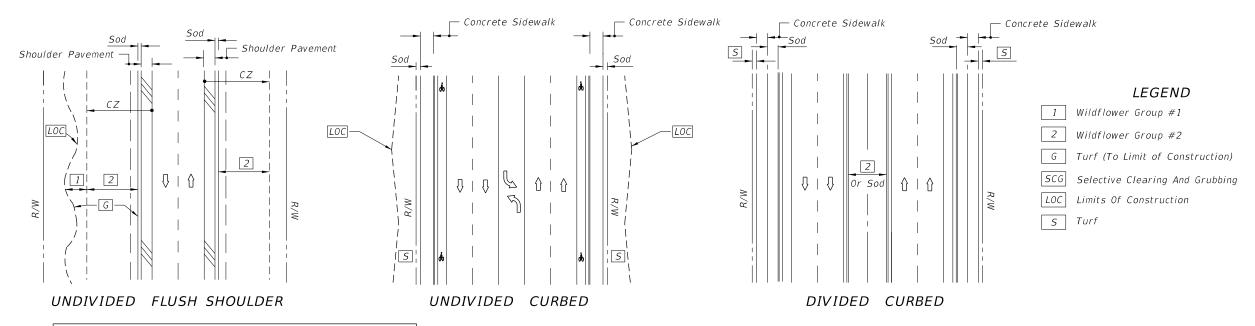
DESCRIPTION:





DIVIDED NARROW MEDIAN WITH OR WITHOUT CURBED MEDIAN

DIVIDED WIDE MEDIAN WITH OR WITHOUT CURBED MEDIAN



WILDFLOWER SEEDING RATES				
Common Name (Botanical Name)	lbs/ac			
#1 Group				
Black-Eyed Susan (Rudbeckia hirta)	2			
Lance-Leaf Tickseed (Coreopsis lanceolata)	10			
Goldenmane Tickseed (Coreopsis basalis)	10			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Softhair Coneflower (Rudbeckia mollis)				
Crimson Clover (Trifolium incarnatum)	15			
#2 Group				
Annual Phlox (Phlox drummondii)	10			
Moss Verbena (Verbena tenuisecta)	6			
Leavenworth's Tickseed (Coreopsis leavenworthii)	10			
Fire Wheel (Gaillardia pulchella)	10			
Crimson Clover (Trifolium incarnatum)	15			
Note: Wildflower seeding rates are for restoring impacted wildflower areas.				

GENERAL NOTES

- 1. All turf establishment shall be performed meeting the requirements of Section 570 of the Standard Specifications.
- 2. Activities such as clearing, grading, and excavating that will disturb one or more acres of land require coverage under the Generic Permit for Stormwater Discharge from Large and Small Construction Activities from the Florida Department of Environmental Protection, and implementation of appropriate pollution prevention measures to minimize erosion and sedimentation and properly manage stormwater.
- 3. Confirm compatibility of wildflower with Seeding Zones.

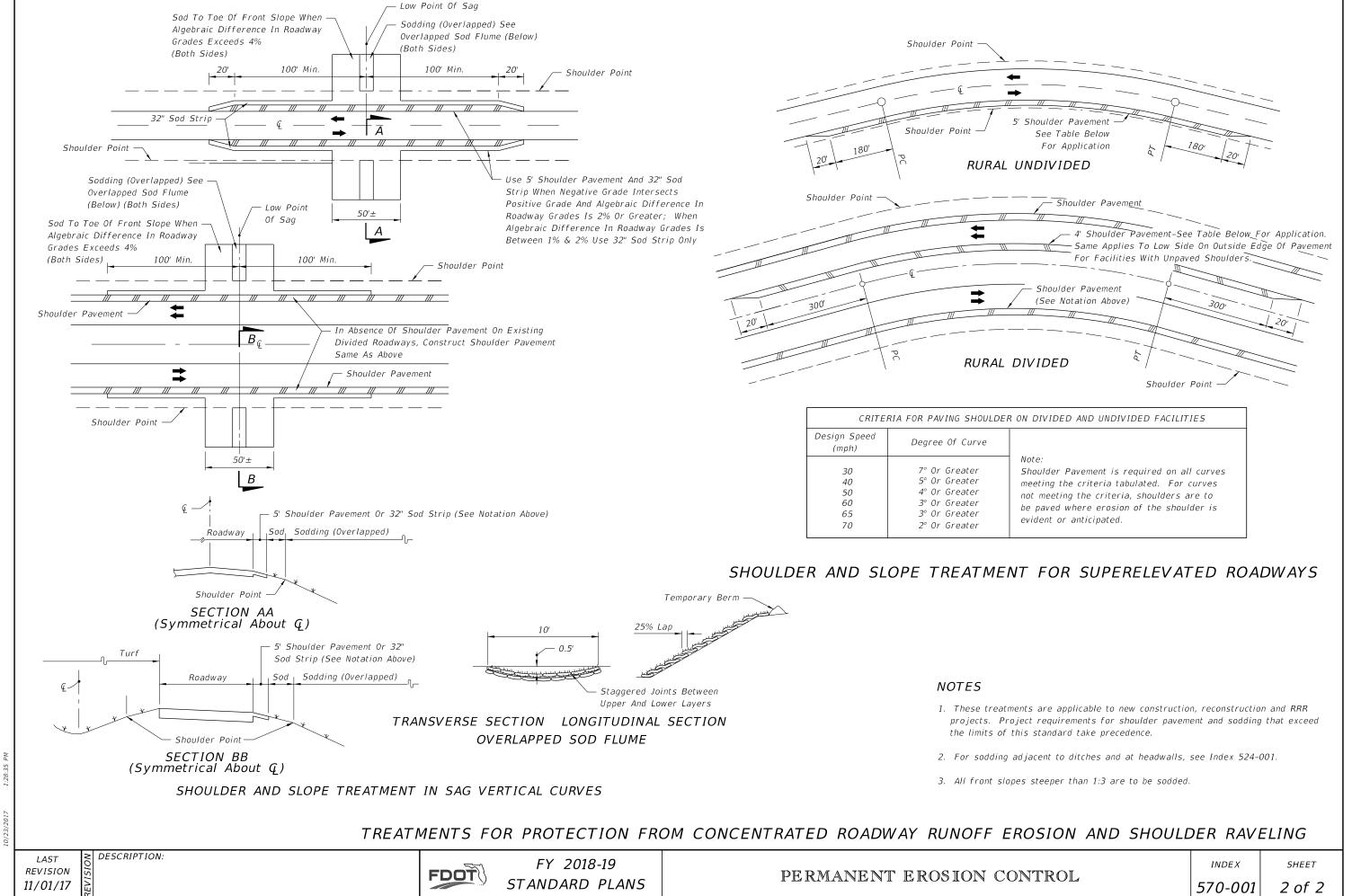
SEEDING ZONES



REVISION 11/01/17

DESCRIPTION:

FDOT

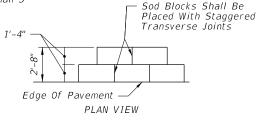


COMPLETED SHOULDER

CRITERIA FOR USING TREATMENT I

Project

- is resurfacing, widening and resurfacing or construction of shoulder pavement
- is rural or is urban without curb and gutter
- resurfacing build-up is less than 3"



PATTERN DETAIL

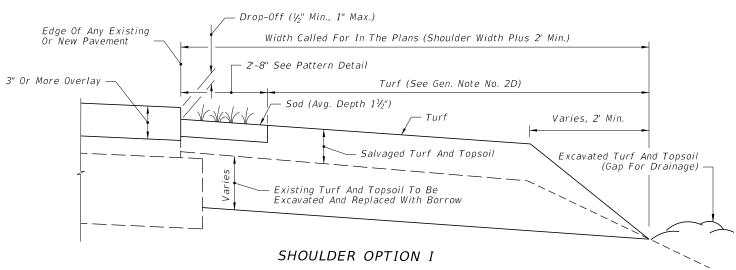
GENERAL NOTES

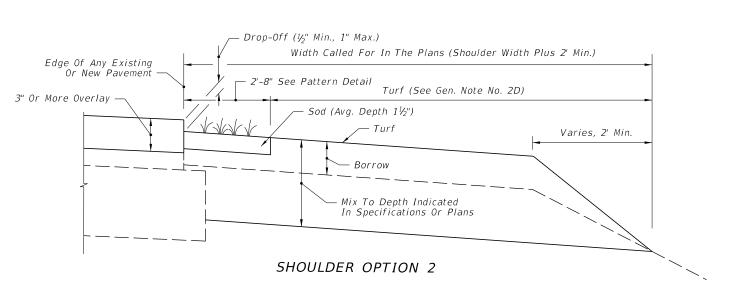
- 1. Treatment I:
 - A. If trenching under sod is necessary to achieve the required Drop-Off, excavated turf and topsoil are to be used for filling voids and low areas at the edge of pavement or for flushing along the edge of sod. Excess material to be uniformly distributed over the shoulder.
 - B. Payment for sod, excavation of turf and topsoil and for back fill of this material under Treatment I is to be included in the contract unit price for Performance Turf, SY. Prepared Soil Layer not required.
- 2. Treatment II.
 - A. All borrow shall meet requirements for a "Select" material in accordance with Index 120-001 and Section 120 of the Standard Specifications.
 - B. Borrow may be used in lieu of excavated turf and topsoil when economically feasible, however the upper 6" shall meet the requirements of Section 162 "Prepared Soil Layer". There will be no additional payment for substituting borrow for excavated turf and topsoil.
 - C. When existing turf and topsoil do not meet the requirements of Section 162 "Prepared Soil Layer", provide additive materials as necessary in the upper 6" to meet the requirements of Section 162. There will be no additional payment for additives.
 - D. Payment for Treatment II will be under Prepared Soil Layer. Sod and other materials for turf establishment shall be paid for as Performance Turf, SY.
- 3. Special attention is to be directed at achieving the required Drop-Off at the edge of pavement, within the dimension range shown.
- 4. Activities such as clearing, grading, and excavating that will disturb one or more acres of land require coverage under the Generic Permit for Stormwater Discharge from Large and Small Construction Activities from the Florida Department of Environmental Protection, and implementation of appropriate pollution prevention measures to minimize erosion and sedimentation and properly manage stormwater.
- 5. Turf Establishment:

DESCRIPTION:

- A. Wildflowers destroyed by shoulder sodding and turf operations are to be reestablished under the seeding rates prescribed for permanent wildflower #2 Group shown by table on Index 570-001.
- B. All turf establishment shall be performed meeting the requirements of Section 570 of the Standard Specifications.

TREATMENT II





CRITERIA FOR USING TREATMENT II

Project

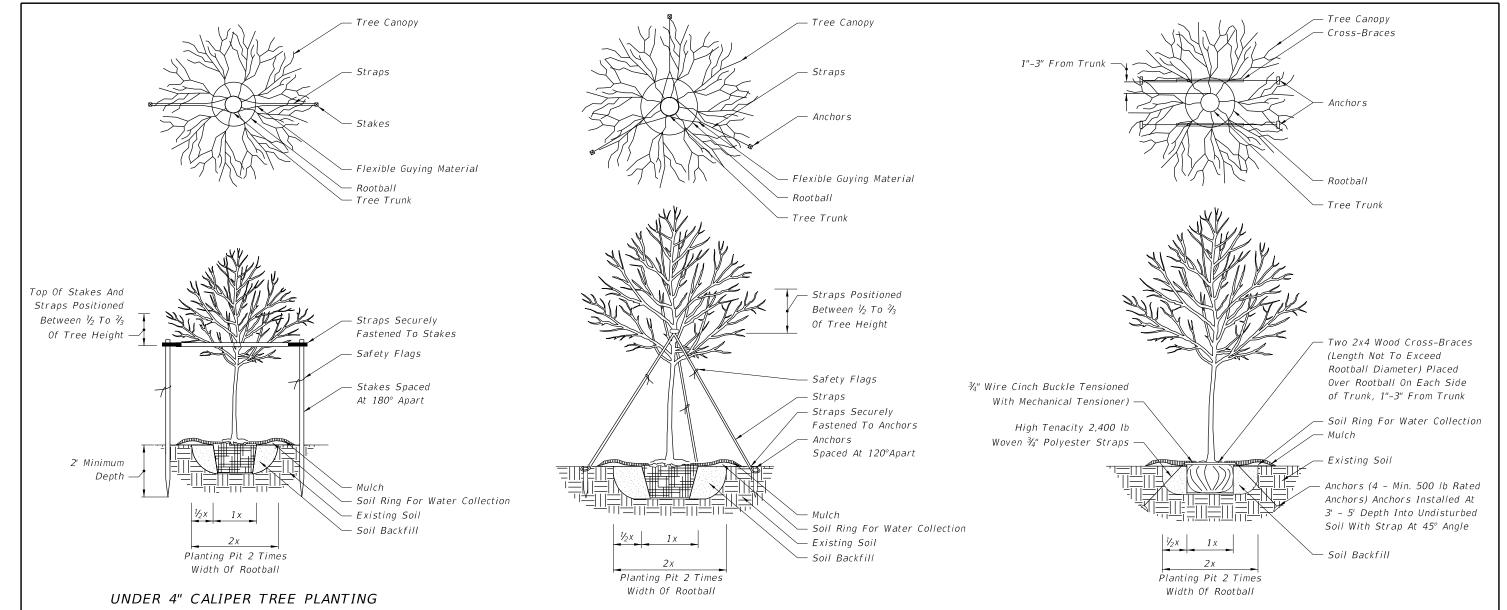
- is resurfacing or construction of shoulder pavement
- is rural or is urban without curb and gutter
- resurfacing build-up is 3" or more

A SIMILAR TREATMENT MAY BE USED FOR PROJECTS THAT REQUIRE SHOULDER WIDENING. DETAILS ARE TO BE SHOWN IN THE PLANS.

1.28.36 PM

10/23/2017





4" AND LARGER CALIPER TREE PLANTING

UNDER 4" CALIPER TREE PLANTING WITH UNDERGROUND BRACING

GENERAL NOTES:

- 1. All dimensions 6" and less are exaggerated for illustrative purposes only.
- 2. Plant containers shall be removed prior to planting. If plants are not container grown, remove a minimum of the top $\frac{1}{3}$ of burlap, fabric, or wire mesh. Never lift or handle the tree by the trunk.
- 3. The uppermost root on all trees shall be covered by less than 1" of soil. Use hand tools to carefully remove all excess soil. The top of root ball shall be set 1"-2" above finish grade after settling and set plumb to the horizon. If planting pit is too deep, remove the tree and firmly pack additional soil in the bottom of the planting pit to raise the rootball. After positioning the tree in the planting pit, slice through rootballs with 3 or 4 vertical slices (top to bottom) equally distributed around the tree.
- 4. Backfill shall be loosened existing soil. Remove rocks, sticks, or other deleterious material greater than 1" in any direction prior to backfilling. Water and tamp to remove air pockets. If existing soils contain excessive sand, clay, or other material not conducive to proper plant growth, contact Engineer prior to planting.
- 5. Soil rings shall be constructed of existing soil at the outer edge of the planting pit, with a height of 3" and gently sloping sides. Do not pile soil on top of rootball.

- 6. Mulch shall be a 3" deep layer placed 2" off the edge of the trunk flare, around the base of shrub, or solidly around groundcover. Never pile mulch against the tree trunk.
- 7. Straps shall be minimum 1" wide nylon or polypropylene. Check straps monthly and adjust as required to eliminate girdling of tree. All wood stakes or anchors shall be located beyond the edge of soil ring in undisturbed soil and located below finished grade, unless otherwise specified.
- 8. Sabal Palms may be hurricane cut. All other palms must have fronds tied with biodegradable twine. Palm trunks shall have no burn marks, scars, or sanding.
- 9. All dimensions provided for wood materials are nominal.
- 10. When a permanent, subsurface, or drip irrigation system is provided, a soil ring is not required. Mulch to edge of planting pit.
- 11. Alternate tree bracing and guying systems specified or approved by the Engineer may be used in lieu of the tree bracing and guying methods detailed on the Index.
- 12. Remove above ground guying systems at the end of the establishment period.

REVISION 11/01/17

DESCRIPTION:

FDOT

FY 2018-19 STANDARD PLANS

LANDSCAPE INSTALLATION

INDEX

SHEET

580-001 1 of 2

