## FINAL LOCATION HYDRAULICS REPORT

## Florida Department of Transportation

## District Four

C.R. 510 Project Development and Environment (PD\&E) Study Limits of Project: C.R. $510 / 85$ Street from C.R. 512 (M.P. 0.0) to 58 Ave (M.P. 5.283)

Indian River County, Florida
Financial Management Number: 405606-2-22-02
ETDM Number: 14233

The environmental review, consultation, and other actions required by applicable federal environmental laws for this project are being, or have been, carried out by FDOT pursuant to 23 U.S.C. § 327 and a Memorandum of Understanding dated December 14, 2016 and executed by FHWA and FDOT.


## EXECUTIVE SUMMARY

The purpose of this report is to address the 100-year (base) floodplain encroachments resulting from the roadway improvements evaluated in the Project Development and Environment (PD\&E) study for CR-510 from CR-512 to 58th Avenue. In accordance with Executive Order 11988m "Floodplain Management", USDOT Order 5650.2, "Floodplain Management Protection", and Federal-Aid Policy Guide 23 CFR 650A, floodplains must be protected. The intent of these regulations is to avoid or minimize highway encroachments within the base floodplains, and to avoid supporting land use development incompatible with floodplain values.

The scope of this project includes reconstruction of CR-510 from an existing two-lane roadway facility to one with four lanes. and replacement of three (3) bridges and two (2) culverts. This project is located in Indian River County, Florida, in the vicinity of the City of Sebastian. Also, the project is located in Township 31 S - Range 38E - Sections 23, 25, and 26 \& Township 31 S - Range 39 E - Sections 21 and 30.

A review of the Flood Insurance Rate Map (FIRM) published by Federal Emergency Management Agency (FEMA), indicates that the study area is located in Special Flood Zones A, AE, X and X500. The portion of the study area located in Zone A has a $1 \%$ annual chance of being flooded by the base flood (100-year storm) with no base flood elevation determined. Areas located in Zone AE also has a $1 \%$ annual chance of being flooded by the 100-year storm with base flood elevations determined. Areas identified in Zones X \& X500 are estimated to have less than 1 foot or no flooding at all during the 100-year storm.

The northbound/southbound portion of CR-510, between CR-512 and just north of 86th St, is located in Zone AE with base elevations ranging from 18.5 to 20.0-ft NAVD ( 20.0 to $21.5-\mathrm{ft} \mathrm{NGVD}$ ). The eastbound/westbound portion of CR-510, located between 90th Avenue and 58th Avenue, is mainly located in Zone $X$. A negligible portion of the eastbound/westbound corridor is located in Zones A and AE. The portion of CR-510 located in Zone AE has a base flood elevation of 15.5ft NAVD (17.0-ft NGVD).

Although this project involves work within the horizontal limits of the 100-year floodplain, no work is being performed below the 100-year flood elevation and, as a result, this project does not encroach upon the base floodplain.

This project will affect several cross drains due to the widening of the CR-510 roadway. Recommendations for culvert replacement or extension are given in Section 5 of the report. It is necessary to ensure that adequate hydraulic capacity and no adverse effects on downstream and upstream canal stages.

## TABLE OF CONTENTS

EXECUTIVE SUMMARY ..... i
TABLE OF CONTENTS ..... ii
LIST OF TABLES ..... iii
LIST OF FIGURES ..... iii
LIST OF APPENDICES ..... iv
1 INTRODUCTION ..... 1-1
1.1 PROJECT DESCRIPTION ..... 1-1
1.2 PROJECT LOCATION ..... 1-2
2 LAND USE ..... 2-1
2.1 Existing Land Use ..... 2-1
2.2 Future Land Use ..... 2-1
3 EXISTING CONDITIONS ..... 3-1
3.1 Hydrologic Data ..... 3-1
3.2 Base Floodplain ..... 3-2
3.3 Soil Properties ..... 3-3
4 EXISTING DRAINAGE BASINS ..... 4-1
4.1 Basin 1 ..... 4-1
4.2 Basin 2 ..... 4-1
4.3 Basin 3 ..... 4-1
4.4 Basin 4 ..... 4-1
4.5 Basin 5 ..... 4-2
4.6 Basin 6 ..... 4-2
4.7 Basin 7 ..... 4-2
4.8 Basin 8 ..... 4-2
4.9 Basin 9 ..... 4-2
4.10 Basin 10 ..... 4-2
5 EXISTING CROSS DRAINS (CD) ..... 5-1
5.1 Culvert 1 (CD 1) ..... 5-1
5.2 Culvert 2 (CD 2) - Box Culvert ..... 5-1
5.3 Culvert 3 (CD 3) ..... 5-1
5.4 Culvert 4 (CD 4) ..... 5-1
6 WATERBODY CROSSINGS (WC) ..... 6-1
6.1 WC 1 (Bridge No. 880047) ..... 6-1
6.2 WC 2 (Bridge No. 880063) ..... 6-1
6.3 WC 3 (Bridge No. 880044) ..... 6-1
6.4 WC 4 (Culvert Replaced with Bridge Structure) ..... 6-1
7 CONCLUSION ..... 7-1

## LIST OF TABLES

Table 3-1: SJRWMD Average Rainfall Data ..... 3-1
Table 3-2: Floodplain Summary ..... 3-3
LIST OF FIGURES
Figure 1-1: Urban typical section ..... 1-1
Figure 1-2: Sub-Urban typical section ..... 1-1
Figure 1-3: Project Area ..... 1-2
Figure 3-1: FEMA Flood Zones ..... 3-2
Figure 4-1: Drainage Basins ..... 4-3
Figure 4-2: Proposed Offsite Drainage System ..... 4-4
Figure 6-1: Cross Drains and Waterbody Crossings ..... 6-2

## LIST OF APPENDICES

## Appendix A: Typical Sections

A1-A5: Recommended Alternative Typical Sections

## Appendix B: Zoning Maps

B1-B5: Indian River County Existing Zoning Maps
B6: $\quad 2030$ Indian River County Future Land Use Map

Appendix C: Rainfall Maps
C1-C9: $\quad$ SJRWMD Average Rainfall Maps

Appendix D: Pond Siting Process
D1-D6: FEMA FIRM

Appendix E: Geotechnical Data
E1-E12: CR 510 Geotechnical Report
E13-E18: NRCS Soil Map

## 1 INTRODUCTION

The Florida Department of Transportation (FDOT) is conducting a Project Development and Environment (PD\&E) Study for the widening and reconstruction of CR-510 from CR-512 to 58th Avenue. The purpose of this report is to address the 100-year (base) floodplain encroachments resulting from the roadway improvements evaluated in the Project Development and Environment (PD\&E) study for CR-510 from CR-512 to 58th Avenue. In accordance with Executive Order 11988m "Floodplain Management", USDOT Order 5650.2, "Floodplain Management Protection", and Federal-Aid Policy Guide 23 CFR 650A, floodplains must be protected. The intent of these regulations is to avoid or minimize highway encroachments within the base floodplains, and to avoid supporting land use development incompatible with floodplain values.

### 1.1 PROJECT DESCRIPTION

The scope of this project includes reconstruction of CR-510 from an existing two-lane roadway facility to one with four lanes. The project was broken down into four (4) distinct segments. Each segment has unique characteristics as well as potential differences in right-of-way, operational, geometric and environmental features. These segments consist of Urban and/or Sub-Urban typical sections. A representation of each section is shown in Figures 1-1 and 1-2 below. Refer to Appendix A for typical sections for the recommended alternative. Also, the scope of this project involves the replacement of three (3) bridges and two (2) culverts.


Figure 1-1: Urban typical section


Figure 1-2: Sub-Urban typical section

### 1.2 PROJECT LOCATION

This project is located in Indian River County, Florida, in the vicinity of the City of Sebastian. Also, the project is located in Township 31 S - Range 38E-Sections 23, 25, and 26 \& Township 31 S Range 39 E - Sections 21 and 30. Refer to Figure 1-3 for the location of the project.


Figure 1-3: Project Area

## 2 LAND USE

### 2.1 Existing Land Use

According to Indian River County Zoning Maps, the project is located in the following zoning areas: Agriculture, Limited Commercial, Public Land Conversation, Planned Development Traditional Neighborhood Design, Multi-Family Residential, and Single-Family Residential. Proposed roadway improvements will occur in all of the zoning area except for Public Land Conversation. Refer to Appendix B for Indian River County Existing Zoning Maps.

### 2.2 Future Land Use

According to the 2030 Indian River County Future Land Use Map, revised on January 12, 2016, future land uses in the project area will remain unaltered with the exception of an agricultural zoning area north of CR 510, between Shakespeare St and $66^{\text {th }}$ Ave. This area has been designated as future residential land. See Appendix B for the 2030 Indian River County Future Land Use Map.

## 3 EXISTING CONDITIONS

### 3.1 Hydrologic Data

Historical rainfall data was obtained from the St John's River Water Management District (SJRWMD) Average Rainfall Maps. See Table 3-1 below for average rainfall for various rainstorm events within the project area and Appendix C for the SJRWMD Average Rainfall Maps.

| Table 3-1: SJRWMD Average Rainfall Data |  |
| :---: | :---: |
| Rainstorm <br> Event | Rainfall <br> (inches) |
| 10-Year 24-Hour | 7.6 |
| $25-$-Year 24-Hour | 9.4 |
| 100 -Year 24-Hour | 12.2 |
| 10 -Year 48-Hour | 8.7 |
| $25-$ Year 48-Hour | 10.5 |
| 100 -Year 48-Hour | 13.6 |
| 10 -Year 96-Hour | 9.8 |
| 25 -Year 96-Hour | 11.9 |
| 100 -Year 96-Hour | 15.2 |

### 3.2 Base Floodplain

The Federal Emergency Management Agency (FEMA) website was reviewed to find the latest Flood Insurance Rate Maps (FIRM) for the project area. FIRM Community Panel Numbers 12061C0078 F, 12061C0087 F and 12061C0090 F dated December 4, 2012, indicates that a small portion of the study area is located in the 100-year floodplain. See Appendix D for FEMA FIRM and Figure 3-1 below for the FEMA flood zones within the project limits.

A review of the Flood Insurance Rate Map (FIRM) published by Federal Emergency Management Agency (FEMA), indicates that the study area is located in Special Flood Zones A, AE, X and X500. The portion of the study area located in Zone A has a $1 \%$ annual chance of being flooded by the base flood (100-year storm) with no base flood elevation determined. Areas located in Zone AE also has a $1 \%$ annual chance of being flooded by the 100 -year storm with base flood elevations determined. Areas identified in Zones X \& X500 are estimated to have less than 1 foot or no flooding at all during the 100-year storm.


Figure 3-1: FEMA Flood Zones
The northbound/southbound portion of CR-510, between CR-512 and just north of 86th St, is located in Zone AE with base elevations ranging from 18.5 to 20.0-ft NAVD ( 20.0 to $21.5-\mathrm{ft} \mathrm{NGVD}$ ). The eastbound/westbound portion of CR-510, located between 90th Avenue and 58th Avenue, is mainly located in Zone X. A negligible portion of the eastbound/westbound corridor is located
in Zones $A$ and AE. The portion of CR-510 located in Zone AE has a base flood elevation of 15.5ft NAVD (17.0-ft NGVD). See Table 3-2 below for a summary of the floodplain for various locations within the study area.

| Table 3-2: Floodplain Summary |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section Name | Road Section Description | Flood <br> Zone | Base El. (ft-NGVD) | Base El. (ft-NAVD) | Existing El. ${ }^{1}$ (ft-NAVD) | Proposed Roadway Recommendation |
| 1 | CR-510 from CR-512 to $86^{\text {th }} \mathrm{St}$ | $\begin{aligned} & \text { AE, X, } \\ & \text { X500 } \end{aligned}$ | $\begin{aligned} & 20.0 \text { to } \\ & 21.5 \end{aligned}$ | $\begin{aligned} & 18.5 \text { to } \\ & 20.0 \end{aligned}$ | $\begin{aligned} & \text { Min } 18.5 \text { to } \\ & 20.0-\mathrm{ft} \end{aligned}$ | Maintain existing elevation of $18.5-\mathrm{ft}$ |
| 2 | CR-510 from 86 ${ }^{\text {th }}$ St to Lateral Canal L | X | - | - | - | Maintain existing elevation |
| 3 | CR-510 from Lateral Canal L to 75 Ct | $\begin{aligned} & \text { AE, X, } \\ & \text { X500 } \end{aligned}$ | 17.0 | 15.5 | Min 17.0-ft | Maintain existing elevation of $17.0-\mathrm{ft}$ |
| 4 | CR-510 from 75 Ct to Paladin Square | X | - | - | - | Maintain existing elevation |
| 5 | CR-510 from Paladin Square to $58^{\text {th }}$ Ave | A, X | - | - | - | Maintain existing elevation |

${ }^{1}$ Existing elevations were obtained from Topo CADD files.
All elevations are based on the NAVD88 datum. To convert to NGVD29 datum, a shift of (+)1.463 feet is needed, as found by using the National Geodetic Survey VERTCON online tool. The proposed roadway edge of pavement will vary within the project area. To ensure that the roadway remains open to traffic during the 100-year flood, minimum elevations were proposed in Table 3-2 above.

No adverse impacts are anticipated to the floodplain, as required by the SJRWMD permitting process. Encroachments due to the proposed roadway improvements are being compensated within the proposed stormwater management system, through the use of wet-detention and dry retention ponds.

### 3.3 Soil Properties

Geotechnical data was obtained from a Geotechnical Report prepared for this study. See Appendix E and Table 3-2 for the soil properties. A review of the documentation indicates that percolation rates along the project corridor are fairly poor with values ranging from 1.15 to 9.69 $\mathrm{E}-05 \mathrm{cfs} / \mathrm{ft}^{2}$. Exfiltration trenches will not be proposed for this project. However, the hydraulic conductivity values are reported for future reference, if needed.

Based on a desktop review of the U.S. Department of Agriculture (USDA) soil map for Indian River County, nine (9) general mapping units underlie the existing and potential pond sites in the study area. Drainage properties of soils within the project area range from poor to moderately well. Refer to Appendix E for soil types.

Table 3-2: Soil Properties

| Map Unit <br> Symbol | Map Unit Name | Description |
| :---: | :--- | :--- |
| 3 | EauGallie fine sand | Hydrologic Soil Group A/D poorly drained |
| 5 | Myakka- Myakka, wet, fine <br> sands, 0 to 2 percent slopes | Hydrologic Soil Group A/D poorly drained |
| 6 | Oldsmar fine sand | Hydrologic Soil Group A/D poorly drained |
| 10 | Riviera Fine sand, 0 to 2 <br> percent slopes | Hydrologic Soil Group A/D poorly drained |
| 12 | Archbold sand, 0 to 5 percent <br> slopes | Hydrologic Soil Group A moderately well <br> drained |
| 13 | Wabasso fine sand | Hydrologic Soil Group C/D poorly drained |
| 14 | Winder fine sand, 0 to 2 <br> percent slopes | Hydrologic Soil Group C/D poorly drained |
| 16 | Pineda fine sand | Hydrologic Soil Group C/D poorly drained |
| 51 | Riviera fine sand, depressional, <br> 0 to 1 percent slopes | Hydrologic Soil Group A/D very poorly <br> drained |

## 4 EXISTING DRAINAGE BASINS

The project area was divided into ten (10) main drainage systems (i.e. Basin 1 to 10), as shown in Figure 4-1. Basin numbers were assigned in increments of 1, starting from the basin at the project begin location. These basins have a combined total area of 145 Acres. Basin delineation was primarily based on existing drainage divides such as canals, intersections, and other high points.

Based on a Desktop Analysis and Field Review, existing offsite properties are draining into the County's Right-of-Way for basins 1, 2, 3, 5, 6, 7, 8, 9 and 10. To prevent comingling of offsite runoff with onsite runoff, a secondary swale or offsite drainage system is proposed. Two different types of offsite drainage systems are proposed. For basins 1, 2, 5, 6, 7 and a portion of 8, existing homes are not close to the Right of Way line. As such, an Open Swale Offsite Drainage System with a 4 feet bottom width, $1 \mathrm{~V}: 4 \mathrm{H}$ side slopes, located in a 32 feet wide easement is proposed for these areas. For basins 3,9 and a portion of 8 , there are existing homes near the proposed CR-510 Right of Way. As such, a 48 inch-wide Trunkline Offsite Drainage System, located in a 20 feet easement, is proposed for these areas. See Figure 4-2 for the location of the proposed offsite drainage.

### 4.1 Basin 1

Basin 1 covers CR-510 from CR-512 to 600-ft. south of Stone Point Drive. The drainage area is approximately 3.17 Acres in size. The total impervious area is approximately 2.09 Acres. A review of previous drainage plans and existing permit documentation shows that runoff is being conveyed towards the north via roadside swales, pipes and culverts, and outfalls to Pond D.

### 4.2 Basin 2

Basin 2 covers CR-510 from 600-ft. south of Stone Point Drive to 89th Street. The drainage area is approximately 6.58 Acres in size. The total impervious area is approximately 3.41 Acres. A review of previous drainage plans and existing permit documentation shows that runoff from the north and south portion (with respect to $400-\mathrm{ft}$. north of Mako Way) of the basin is being conveyed by roadside swales, pipes and culverts and ultimately discharges into Lateral Canal D. In addition, runoff from the southwest portion of the basin discharges into Lake A.

### 4.3 Basin 3

Basin 3 covers CR-510 from 89th Street to 85th Street. The drainage area is approximately 7.65 Acres in size. The total impervious area is approximately 3.12 Acres. A review of the previous drainage plans and existing permit documentation shows that runoff from the left side of the north portion of this basin (with respect to 86th St ) is being conveyed to Lake A via roadside swales and sheet flow. Runoff from all other areas within this basin, is conveyed towards the south and ultimately discharges into Lateral Canal D.

### 4.4 Basin 4

Basin 4 covers CR-510 from Lateral Canal D to 86th Ave. The drainage area is approximately 6.69 Acres in size. The total impervious area is approximately 2.73 Acres. A review of the previous drainage plans and permit documentation shows that runoff from this basin is being conveyed
toward the west by roadside swales, pipes and culverts and ultimately discharges into Lateral Canal D. Currently, there are existing connections to two wet detention ponds of Bluewater Bay PUD for treatment before discharging into the South Prong of the Sebastian River.

### 4.5 Basin 5

Basin 5 covers CR-510 from 86th Ave to Lateral Canal C. The drainage area is approximately 10.14 Acres in size. The total impervious area is approximately 1.93 Acres. A review of the previous drainage plans shows that runoff from this basin is being conveyed toward the east by roadside swales, pipes and culverts and discharges directly into Lateral Canal C.

### 4.6 Basin 6

Basin 6 covers CR-510 from Lateral Canal C to Lateral Canal L. The drainage area is approximately 10.03 Acres in size. The total impervious area is approximately 1.91 Acres. A review of the previous drainage plans shows that runoff from this basin is being conveyed toward the west and east by roadside swales, pipes and culverts. Runoff from the west portion of the basin (with respect to $900-\mathrm{ft}$. west of 79 th Ter) discharges directly into Lateral Canal C and runoff from the east portion of the basin discharges directly into Lateral Canal L.

### 4.7 Basin 7

Basin 7 covers CR-510 from Lateral Canal L to approximately $2,500-\mathrm{ft}$. west of 70 th Ave. The drainage area is approximately 10.22 Acres in size. The total impervious area is approximately 1.95 Acres. A review of the previous drainage plans shows that runoff from this basin is being conveyed toward the west by roadside swales, pipes and culverts and discharges directly into Lateral Canal L.

### 4.8 Basin 8

Basin 8 covers CR-510 from approximately 2,500-ft. west of 70 th Ave to 66 th Ave. The drainage area is approximately 20.63 Acres in size. The total impervious area is approximately 3.93 Acres. Runoff from this basin is being conveyed toward the west by roadside swales, pipes and culverts and ultimately discharges into Lateral Canal L.

### 4.9 Basin 9

Basin 9 covers CR-510 from 66th Ave to 62nd Ave. The drainage area is approximately 6.30 Acres in size. The total impervious area is approximately 1.94 Acres. Runoff from this basin is being conveyed toward the west by roadside swales, pipes and culverts and ultimately discharges into Lateral Canal L.

### 4.10 Basin 10

Basin 10 covers CR-510 from 62th Ave to 58th Ave. The total impervious area is approximately 2.49 Acres. Runoff from this basin is being conveyed by roadside swales, pipes and culverts and discharges into Sub-Lateral G-S canal.


Figure 4-1: Drainage Basins


Figure 4-2: Proposed Offsite Drainage System

## 5 EXISTING CROSS DRAINS (CD)

There are four (4) identified cross drains in this project. See below for a description of each cross drain. Refer to Figure 6-1 for a map of the cross drains within the project area.

### 5.1 Culvert 1 (CD 1)

Culvert 1 (CD 1) is a 48 " Reinforced Concrete Pipe (RCP), with a length of $102-\mathrm{ft}$., that runs under CR-510 approximately 600 ft . south of Stony Point Drive. This culvert is used to connect Lateral D Canal Watershed to Sebastian River Watershed. Proposed conditions, under the recommended alternative, include the widening of CR-510 on both sides of the roadway. To maintain the regional drainage pattern, an extension of CD 1 on both sides will be needed. CD 1 will should be extended $15-\mathrm{ft}$ towards the west and $2-\mathrm{ft}$ toward the east.

### 5.2 Culvert 2 (CD 2) - Box Culvert

Culvert 2 (CD 2) is a box culvert that is located at the intersection of CR-510 and 89th Street. It has a length of $106-\mathrm{ft}$., depth of $10-\mathrm{ft}$., and a width of $18-\mathrm{ft}$. to the west and $14-\mathrm{ft}$. to the east. This culvert was designed to connect Lateral D Canal Watershed to Sebastian River Watershed. Proposed conditions, under the recommended alternative, involve realignment and widening of CR-510 towards the east. To maintain the regional drainage pattern, a culvert replacement will be needed. A box culvert of length $121-\mathrm{ft}$. will be required.

### 5.3 Culvert 3 (CD 3)

Culvert 3 (CD 3) is a 36 " Corrugated Metal Pipe (CMP) with a length of $42-\mathrm{ft}$. This culvert runs under CR-510 and is located approximately 977 ft . east of 75 th Ct . Culvert 3 (CD 3) was designed to connect the Lateral L Canal Watershed to the Sebastian River Watershed. The proposed roadway alignment and widening is towards the north. To maintain the regional drainage pattern, a culvert replacement will be needed. A culvert of length $134-\mathrm{ft}$ will be required for the recommended alternative.

### 5.4 Culvert 4 (CD 4)

Culvert 4 (CD 4) has a north pipe of 24 " Reinforced Concrete Pipe (RCP) with a length of 42ft and a south pipe of $24^{\prime \prime}$ Corrugated Metal Pipe (CMP) with a length of $41-\mathrm{ft}$. This culvert runs under CR-510 and is located approximately 960 ft . west of 58th Ave. Culvert 4 (CD 4) was designed to connect the Lateral G Canal Watershed to the Sebastian River Watershed. For the recommended alternative, the proposed roadway alignment and widening is towards the south. To maintain the regional drainage pattern, a culvert replacement will be needed. A culvert of length 104 -ft will be required.

## 6 WATERBODY CROSSINGS (WC)

There are four (4) identified waterbody crossings in this project. Roadway improvements will involve work within the jurisdiction of St John River Florida Water Management District (SJRWMD), Sebastian River Water Improvement District (SRID) and Indian River Farms Water Control District (IRFWCD). See below for a description of the waterbody crossings within the project area. Refer to Figure 6-1 for a map of the waterbody crossings within the project area.

### 6.1 WC 1 (Bridge No. 880047)

Existing Bridge 880047 is located at the intersection of CR-510 and approximately 233 ft . east of 90th Avenue. This bridge carries CR-510 eastbound and westbound traffic over Lateral D Canal. FDOT is proposing to place a culvert crossing at Lateral Canal-D to accommodate the proposed roadway alignment, with a wider horizontal curve. The new curve will improve the turning radius for traffic safety. Existing Bridge 880047 will be removed for the recommended alternative and a triple-box culvert will be installed at Lateral D Canal where realigned CR-510 crosses the canal. Preliminary calculations indicate that a triple box culvert and the existing bridge will provide a similar hydraulic flow capacity since required hydraulic capacity with the allowable backwater increment of 0.1-ft., a triple box culvert is being proposed to address potential clogging and maintenance concerns. The proposed culvert provides a minimum drift clearance of $1-\mathrm{ft}$. above the $100-\mathrm{Year}$ FEMA elevation.

### 6.2 WC 2 (Bridge No. 880063)

Existing Bridge 880063 is located at the intersection of CR-510 and approximately 79 ft . west of 82nd Avenue. This bridge carries CR-510 eastbound and westbound traffic over Lateral C Canal. Existing Bridge 880063 will be removed for the recommended alternative and a new bridge will be installed over Lateral C Canal to accommodate the proposed widening of CR-510. Existing plans show that the bridge at this location consists of timber and steel piles with an overall length of $75-\mathrm{ft}$. The bridge proposed for this location will have an overall length of $85-\mathrm{ft}$.

### 6.3 WC 3 (Bridge No. 880044)

Existing Bridge 880044 located at the intersection of CR-510 and approximately 433 ft . east of 79th Terrace. carries CR-510 eastbound and westbound traffic over the Lateral L Canal. Existing Bridge 880044 will be removed for the recommended alternative and a new bridge will be installed over Lateral L Canal to accommodate the proposed widening of CR-510.

### 6.4 WC 4 (Culvert Replaced with Bridge Structure)

The existing $106^{\prime \prime} \times 60^{\prime \prime}$ CMP culvert is currently a cross drain that is used to connect Lateral L Canal to the Sebastian River Watersheds. The existing $106^{\prime \prime} \times 60$ " CMP culvert will be removed for the recommended alternative and replaced with a bridge structure. For more information, refer to the Bridge Hydraulics Report.


Figure 6-1: Cross Drains and Waterbody Crossings

## 7 CONCLUSION

There is no change in flood "Risk" or adverse floodplain impacts associated with this project. The following floodplain statement was taken from Chapter 13 of the FDOT PD\&E Manual (statement Number 1).

Although this project involves work within the horizontal limits of the 100-year floodplain, no work is being performed below the 100-year flood elevation and, as a result, this project does not encroach upon the base floodplain.

This project will affect several cross drains due to the widening of the CR-510 roadway. Recommendations for culvert replacement or extension are given in Section 5 of the report. It is necessary to ensure that adequate hydraulic capacity and no adverse effects on downstream and upstream canal stages.

## APPENDIX A

## (Typical Sections)

A1-A5: Recommended Alternative Typical Sections



| $\text { FDOT\} }$ | Aerial Imagery by: <br> Florida Department of Transportation Dated 2015 | CR 510 PD\&E Study <br> FM No. 405606-2-22-01 | SEGMENT 2 - TYPICAL SECTION WITH ALIGNMENT FEATURES | 0 | 250 | ${ }^{500} \text { Feet }$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Figure 4 - Segment 2 Typical Section with Alignment Features

Figure 5 - Segment 3 Typical Section with Alignment Features



Figure 7 - Typical Section Details

## APPENDIX B

## (Zoning Maps)

B1-B5: Indian River County Existing Zoning Maps

B6: 2030 Indian River County Future Land Use Map



## OFFICIAL ZONING ATLAS

INDIAN RIVER COUNTY, FLORIDA


Map Num 19,2016


Community Development Department
1801 27th Street 1801 27th Street
Vero Beach, FL 32960
Phone: $772-226-1237$

MAP LEGEND


ZONING




## OFFICIAL ZONING ATLAS

INDIAN RIVER COUNTY, FLORIDA
Section 30 31S-39E
Map Number 11.10, 2016
Community Development Depa
180127th Street
Vero Beach FL 32900
Phone: $772-226-1237$
CNADAT dVN

号
0
0
0
0

$\xrightarrow[\text { LOCATOR MAP }]{\text { BREVARD }}$


$\underbrace{2000}_{1 \text { inch }=435 \text { feet }}{ }^{800}{ }_{\text {Feet }}^{800}$


2030 Indian River County Future Land Use Map


## APPENDIX C

(Rainfall Maps)

## C1-C9: SJRWMD Average Rainfall Maps



Figure 6. 10-Year 24-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 7. 25-Year 24-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 8. 100-Year 24-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 9. 10-Year 48-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 10. 25-Year 48-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 11. 100-Year 48-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 12. 10-Year 96-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 13. 25-Year 96-Hour Maximum Rainfall for Northeast Florida, Inches.


Figure 14. 100-Year 96-Hour Maximum Rainfall for Northeast Florida, Inches.

## APPENDIX D

## (Floodplain Maps)

D1-D6: FEMA FIRM






## APPENDIX E

## (Geotechnical Data)

## E1-E12: CR 510 Geotechnical Report

E13-E18: NRCS Soil Map

Date: $\quad$ April 12, 2017
Prepared by: GCME, Inc.
TO: Metric Engineering, Inc.
13940 SW 136th St \#200
Miami, Florida 33186
Attention: Mr. Carlos Rodriguez, P.E.
Transportation Planning Manager
SUBJECT: Geotechnical Report -Revised
Roadway Soils Survey and Structures
CR 510 PD\&E Study
From CR 512 to $58^{\text {th }}$ Avenue
Indian River County, Florida
FM No.: 405606-2-22-02
GCME Project No.: 2000-01-15013

Dear Mr. Rodriguez
GCME, Inc. has completed the Geotechnical Services Report - Roadway Soils Survey and Structures in connection with the subject project. The purpose of this report is to provide all available existing geotechnical information to the roadway/structural engineers and for preparation of the plans for the proposed improvements (PD\&E Study).

This report is updated to incorporate the comments made in reference to our previous report dated December 20, 2016 and the follow up discussion during our meeting with Ms. Maria Formoso, P.E., Mr. Matthew Gisondi, P.E., Mr. Terrance Walters (with FDOT) and you on March 21, 2017, at the FDOT District IV office.

The following report includes the methods of study, evaluations and recommendations concerning geotechnical aspects of the proposed improvements.

The work was completed following our contract with your firm and followed the basic guidelines of the Florida Department of Transportation (FDOT) Soils and Foundations Handbook, 2016. This report is written using English units.











## Table-E1

## Borehole Permeability Test Results

Reference: FDOT Project, FPID No.: 405606-1-22-01

| BHP Name /Location | Station | Offset* | $\begin{gathered} k \\ \left(\mathrm{cfs} / \mathrm{ft}^{2}\right) \end{gathered}$ | k (feet/day) |
| :---: | :---: | :---: | :---: | :---: |
| BHP \# 1 [E5] | 39+00 | 4 RT | $6.94 \mathrm{E}-05$ | 6.0 |
| BHP \# 2 [W10] | 50+00 | 5 LT | 1.16E-05 | 1.0 |
| BHP \# 3 [E27] | 83+00 | 8 RT | $3.47 \mathrm{E}-05$ | 3.0 |
| BHP \# 4 [W32] | 94+00 | 1 LT | $1.15 \mathrm{E}-05$ | 1.0 |
| BHP \# 5 [N51] | $131+00$ | 0 LT | 1.16E-05 | 1.0 |
| BHP \# 6 [S56] | $142+00$ | 2 RT | 3.47E-05 | 3.0 |
| BHP \# 7 [N74] | $177+00$ | 1 LT | 6.14E-06 | 0.5 |
| BHP \# 8 [S89] | 210+00 | 1 RT | 1.23E-05 | 1.1 |
| BHP \# 9 [N97] | 223+00 | 1 LT | $3.50 \mathrm{E}-05$ | 3.0 |
| BHP \# 10 [S101] | 234+00 | 2 RT | 8.02E-05 | 7.0 |
| BHP \# 11 [N121] | 273+00 | 1 LT | 3.62E-05 | 3.1 |
| BHP \# 12 [S127] | 286+00 | 2 RT | 9.69E-05 | 8.4 |
| BHP \# 13 [N145] | $321+00$ | 2 LT | 1.53E-05 | 1.3 |
| BHP \# 14 [S165] | $362+00$ | 5 RT | 2.59E-05 | 2.2 |

* Offset measured from edge of pavement


Soil Map－Indian River County，Florida

| MAP LEGEND |  |  |  | MAP INFORMATION |
| :---: | :---: | :---: | :---: | :---: |
| Area of Interest（AOI） |  | \％ | Spoil Area | The soil surveys that comprise your AOI were mapped at 1：20，000． |
| Soils | Area of Interest（AOI） | 0 | Stony Spot | Please rely on the bar scale on each map sheet for map measurements． |
| $\square$ | Soil Map Unit Polygons | 4 | ery Stony Spot | Source of Map：Natural Resources Conservation Service |
| $\cdots$ | Soil Map Unit Lines | 8 | Wet Spot | Web Soil Survey URL：http：／／websoilsurvey．nrcs．usda．gov |
|  |  | $\triangle$ | Other | Coordinate System：Web Mercator（EPSG：3857） |
|  | Soll | － | Special Line Features | Maps from the Web Soil Survey are based on the Web Mercator |
| （0） | Blowout | Water Fe | ures | projection，which preserves direction and shape but distorts distance and area．A projection that preserves area，such as the |
| 囚 | Borrow Pit | $\sim$ | Streams and Canals | Albers equal－area conic projection，should be used if more accurate calculations of distance or area are required． |
| 餄 | Clay Spot |  |  |  |
| $\bigcirc$ | Closed Dep |  |  | This product is generated from the USDA－NRCS certified data as of the version date（s）listed below． |
|  | Gravel Pit |  |  | Soil Survey Area：Indian River County，Florida |
|  |  | － | US Routes | Survey Area Data：Version 14，Nov 19， 2015 |
|  | Gravelly Spot | $\approx$ | Major Roads |  |
| （6） | Landfill | $\sim$ | Local Roads | or larger． |
| A | Lava Flow | Backgro |  | Date（s）aerial images were photographed：Dec 15，2010－Mar |
| 出 | Marsh or swamp | 5 | Aerial Photography | 13， 2011 |
| 资 | Mine or Quarry |  |  | The orthophoto or other base map on which the soil lines were |
| © | Miscellaneous Water |  |  | compiled and digitized probably differs from the background imagery displayed on these maps．As a result，some minor shifting |
| － | Perennial Water |  |  | of map unit boundaries may be evident． |
| $v$ | Rock Outcrop |  |  |  |
| $+$ | Saline Spot |  |  |  |
| $\because$ | Sandy Spot |  |  |  |
| 든 | Severely Eroded Spot |  |  |  |
| ¢ | Sinkhole |  |  |  |
|  | Slide or Slip |  |  |  |
| （6） | Sodic Spot |  |  |  |


| Table: Soil Types in Project Area |  |  |
| :---: | :---: | :---: |
| Map Unit Symbol | Map Unit Name | Description |
| 3 | EauGallie fine sand | - Hydrologic Soil Group A/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 5 inches consist of fine sand 5 to 26 inches consist of fine sand 26 to 42 inches consist of fine sand 42 to 47 inches consist of fine sand 47 to 62 inches consist of sandy clay loam 62 to 80 inches consist of loamy sand <br> - Permeability rate: 0.06 to $1.98 \mathrm{in} / \mathrm{hr}$ |
| 5 | Myakka- <br> Myakka, wet, <br> fine sands, 0 to <br> 2 percent slopes | - Hydrologic Soil Group A/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 6 inches consist of fine sand 6 to 20 inches consist of fine sand 20 to 36 inches consist of fine sand 36 to 80 inches consist of fine sand <br> - Permeability rate: 0.57 to $5.95 \mathrm{in} / \mathrm{hr}$ |
| 6 | Oldsmar fine sand | - Hydrologic Soil Group A/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 5 inches consist of fine sand 5 to 32 inches consist of fine sand 32 to 50 inches consist of fine sand 50 to 62 inches consist of sandy clay loam 62 to 80 inches consist of loamy fine sand <br> - Permeability rate: 0.06 to $0.20 \mathrm{in} / \mathrm{hr}$ |


| Table: Soil Types in Project Area |  |  |
| :---: | :---: | :---: |
| Map Unit Symbol | Map Unit Name | Description |
| 10 | Riviera fine sand, 0 to 2 percent slopes | - Hydrologic Soil Group A/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 6 inches consist of fine sand 6 to 28 inches consist of fine sand 28 to 36 inches consist of sandy loam 36 to 42 inches consist of sandy clay loam 42 to 80 inches consist of fine sand <br> - Permeability rate: 0.60 to $6.00 \mathrm{in} / \mathrm{hr}$ |
| 12 | Archbold sand, <br> 0 to 5 percent <br> slopes | - Hydrologic Soil Group A <br> - Drainage Class: moderately well drained <br> - Profile: 0 to 2 inches consist of sand 2 to 80 inches consist of sand <br> - Permeability rate: 20.00 to $50.02 \mathrm{in} / \mathrm{hr}$ |
| 13 | Wabasso fine sand | - Hydrologic Soil Group C/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 7 inches consist of fine sand 7 to 24 inches consist of fine sand 24 to 35 inches consist of fine sand 35 to 48 inches consist of sandy clay loam 48 to 80 inches consist of loamy sand <br> - Permeability rate: 0.06 to $0.20 \mathrm{in} / \mathrm{hr}$ |


| Table: Soil Types in Project Area |  |  |
| :---: | :---: | :---: |
| Map Unit Symbol | Map Unit Name | Description |
| 14 | Winder fine sand, 0 to 2 percent slopes | - Hydrologic Soil Group C/D <br> - Drainage Class: poorly drained <br> - Profile: 0 to 7 inches consist of fine sand 7 to 17 inches consist of fine sand 17 to 23 inches consist of sandy loam 23 to 34 inches consist of sandy loam 34 to 48 inches consist of sandy loam 48 to 65 inches consist of sandy loam 65 to 80 inches consist of loamy sand <br> - Permeability rate: 0.06 to $0.20 \mathrm{in} / \mathrm{hr}$ |
| 16 | Pineda fine sand | Hydrologic Soil Group C/D <br> Drainage Class: poorly drained <br> Profile: 0 to 4 inches consist of fine sand <br> 4 to 9 inches consist of fine sand <br> 9 to 23 inches consist of fine sand <br> 23 to 40 inches consist of sandy loam <br> 40 to 80 inches consist of loamy sand <br> Permeability rate: 0.06 to $0.20 \mathrm{in} / \mathrm{hr}$ |
| 51 | Riviera fine sand, depressional, 0 to 1 percent slopes | Hydrologic Soil Group A/D <br> Drainage Class: very poorly drained <br> Profile: 0 to 4 inches consist of fine sand <br> 4 to 36 inches consist of fine sand <br> 36 to 42 inches consist of fine sandy loam <br> 42 to 56 inches consist of fine sand <br> 56 to 80 inches consist of fine sand <br> Permeability rate: 0.60 to $2.00 \mathrm{in} / \mathrm{hr}$ |

## Map Unit Legend

| Indian River County, Florida (FL061) |  |  |  |
| :---: | :---: | :---: | :---: |
| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
| 3 | EauGallie fine sand | 52.5 | 18.5\% |
| 5 | Myakka-Myakka, wet, fine sands, 0 to 2 percent slopes | 13.1 | 4.6\% |
| 6 | Oldsmar fine sand | 38.6 | 13.6\% |
| 10 | Riviera fine sand, 0 to 2 percent slopes | 95.8 | 33.8\% |
| 12 | Archbold sand, 0 to 5 percent slopes | 9.3 | 3.3\% |
| 13 | Wabasso fine sand | 40.5 | 14.3\% |
| 14 | Winder fine sand, 0 to 2 percent slopes | 17.9 | 6.3\% |
| 16 | Pineda fine sand | 8.0 | 2.8\% |
| 51 | Riviera fine sand, depressional, 0 to 1 percent slopes | 3.4 | 1.2\% |
| 99 | Water | 4.5 | 1.6\% |
| Totals for Area of Interest |  | 283.6 | 100.0\% |

