### GENERAL NOTES

- 1. The illustrations for guardrail applications are standard configurations; adjustments are to be made as required by site specific conditions to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figures 1 and 2, and other application details of this Index.
- 3. One Panel (i.e., panel length) equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standard indexes or where specifically called for in the plans.

Post spacing shall be 6'-3" except that reduced spacing shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Detail J and Index No. 402) and transitions to redirective crash cushions, (b) the conditions in Note No. 7 below, (c) special post applications, (d) reduced post spacing required for specific end anchorage assemblies, and, (e) specific spacing called for in the plans.

- 4. The standard guardrail mounting height for W-beam guardrail is 2'-1" and for thrie-beam guardrail is 1'-9" to the center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a deviation of 1" below and 3" above the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines). For guardrail placed on slopes beyond the shoulder point, there shall be no deviation more than 1" below to 3" above the desired height within any 25 foot section of quardrail. For standard guardrail with a mounting height of 2'-1'' to the center of beam, a construction tolerance of  $\frac{1}{2}''$  below and 1" above the standard mounting height is permissible. Use the applicable 2013 Design Standards, Index 400 Series for repair or replacement of existing W-beam guardrail systems with a mounting height of 1'-9" to the center of beam.
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end anchorage for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end anchorages will be constructed only when restraints prevent construction of flared end anchors.

Guardrail end anchorage assemblies shall be of the type called for in the plans. If the plans call for a "flared" end anchorage assembly and does not identify the specific system to be used, the contractor has the option to construct any FDOT approved "flared" end anchorage assembly identified on the Approved Products List (APL), subject to the conditions identified in these drawings, or the approved APL drawings.

If the plans call for a "parallel" end anchorage assembly and does not identify the specific system to be used, the contractor has the option to construct any FDOT approved "parallel" end anchorage assembly identified on the APL, subject to the conditions identified in these drawings, or the approved APL drawings.

If the plans call for a specific end anchorage assembly, substitutions with other end anchorage assemblies will not be permitted unless approved by the Engineer. Approved substitutions will not be eligible for CSIP consideration.

When an end anchorage assembly is attached to guardrail and PEDESTRIAN SAFETY TREATMENTS are required, only end anchorage assemblies approved with timber posts are to be used.

Currently approved proprietary end anchorage assemblies are identified on the Approved Products List (APL). Manufacturers seeking approval of proprietary end anchorage assemblies for inclusion on the APL must submit an application with appropriate documentation showing that the end anchorage assembly is deemed eligible by the Federal Highway Administration (FHWA) for federal funding on the National Highway System (NHS) and is compatible with FDOT guardrail systems. System approvals will be contingent upon FDOT's evaluation of crash test performance results for consistency with FDOT system applications and use. If approved, product drawings signed and sealed by a professional engineer licensed in the State of Florida is required.

- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 5' minimum for standard W-beam, other guardrail configurations with reduced post spacing may be applicable; see General Note No. 11 and the minimum offset table on Sheet 19. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier wall shall be used unless other shielding is approved by the Engineer of Record. See Index No. 410 for safety shape concrete barrier walls and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at roadside hazards or other areas where the Engineer has deemed guardrail necessary, guardrail should be considered on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone and fill heights are 6' or greater. Curbed sections should be evaluated for installation of guardrail where fill slopes are steeper than 1:3 and fill heights are 6' or greater within 22' of the traveled way. For additional details on curbed sections, see DETAIL L, LOCATION AT CURB & GUTTER SECTIONS.
- 9. The guardrail to bridge connections contained in this Index are for bridges with Test Level 4 traffic railing barriers. For guardrail to concrete barrier wall connections see Index No. 410. For existing bridges receiving retrofit traffic railing barriers see Index No. 402

		2015 DESIGN STANDARDS		
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- 10. The W-beam guardrail system in this index is the standard system to be used on the State Highway System where a Test Level 3 semi-rigid barrier is required.
- 11. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic railing barriers, to concrete and certain water filled safety shaped barriers, certain crash cushions and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 402, 410 and 414. The use of thrie-beam guardrail with standard offset blocks (Test Level 3 semi-rigid system) may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
  - a. W-beam deflection is marginal,
- b. W-beam with rub rail considered functionally deficient,
- c. Vehicle overriding W-beam is probable,
- d. Drainage will be impeded or blocked by the use of concrete barrier wall (subject to deflection space requirements),
- e. High frequency of repairs to W-beam,
- f. Spandrel beam with low deflection needed around unrelocatable structure,
- g. Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g., passenger vans and small buses).

The modified thrie-beam guardrail is a Test Level 4 semi-rigid system and may be used where a Test Level 4 guardrail is required.

- 12. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
- a. Wide medians where approach end anchorage is located outside of opposing roadway clear zone,
- b. Medians of uniform width that are occupied by other transportation and joint use facilities,
- c. Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail installations.
- d. Medians of bifurcated roadways.
- 13. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 14. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crash worthiness at terminals. Crash cushions or Redirective Median End Anchorage Assemblies shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 15. Corrugated sheet steel beams, end shoes, end sections and backup plates shall conform to the requirements of AASHTO M180, Class A (12 Gauge), Type II (zinc) coating, except the W-Thrie Beam Transition Panel detailed on Sheet 20A shall be Class B (10 Gauge). All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTO requirements.
- 16. Offset blocks:
  - a. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing COMBINATIONS are tabulated on Sheet 16.
- b. Composite offset block installations shall be constructed on guardrail outside of approach end anchorage assemblies or any transition system connecting to a rigid or thrie-beam barrier.
- 17. New holes in existing guardrail are to be punched. Where necessary to enlarge existing holes to guardrail, the work will be done by drilling or reaming. Repair damaged galvanization in accordance with Section 562. Burning of any holes will not be permitted.
- 18. For BARRIER DELINEATOR see DETAIL M.
- 19. Any run of guardrail with existing concrete posts that is being relocated under a construction or maintenance contract shall be replaced using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 20. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for CSIP consideration.
- 21. On roadways designated for reverse laning, all downstream ends of guardrail that are not shielded or that are not designed as approach end terminals shall be marked with post-mounted Type 3 Object Markers. Trailing bridge ends and trailing shoulder concrete barrier wall ends shall be marked with Type 3 Object Markers except where there is trailing end guardrail. Object markers to be installed facing reverse laning traffic. The cost of the object marker shall be included in the cost of the guardrail.

GUARDRAIL

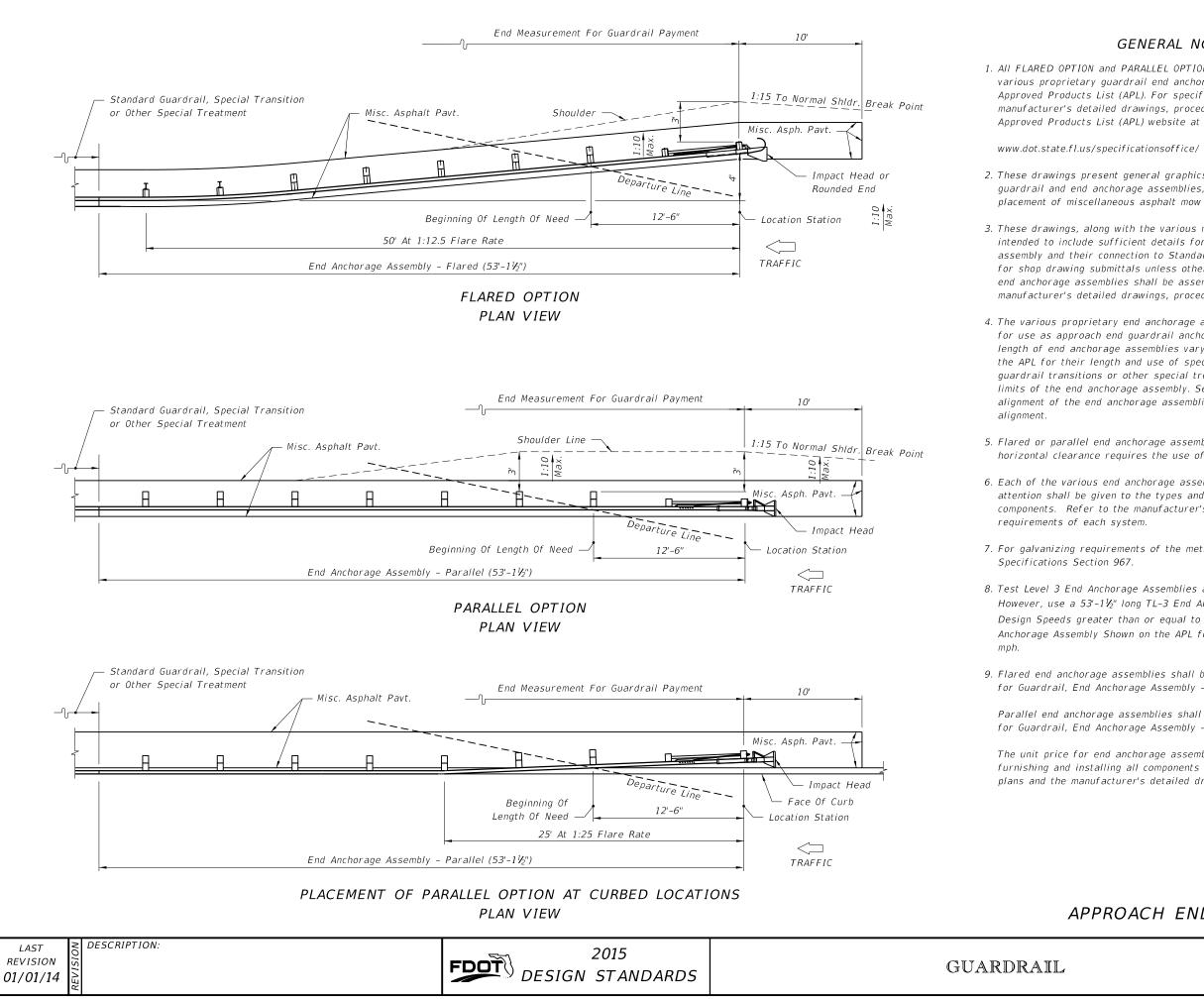
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steel offset blocks may remain throughout the service life of the existing guardrail. PERMISSIBLE POST AND OFFSET BLOCK

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### GENERAL NOTES

1. All FLARED OPTION and PARALLEL OPTION drawings are representative of the various proprietary quardrail end anchorage assemblies listed on the Department's Approved Products List (APL). For specific details and requirements refer to the manufacturer's detailed drawings, procedures and specifications located on the

2. These drawings present general graphics which depict the limits of payment for guardrail and end anchorage assemblies, modifications to the shoulder, and placement of miscellaneous asphalt mow strips.

3. These drawings, along with the various manufacturer drawings on the APL, are intended to include sufficient details for installation of the end anchorage assembly and their connection to Standard Guardrail. This precludes requirements for shop drawing submittals unless otherwise called for in the plans. The various end anchorage assemblies shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.

4. The various proprietary end anchorage assemblies listed on the APL are intended for use as approach end guardrail anchorages for Standard Guardrail. The actual length of end anchorage assemblies vary-refer to the manufacturer's drawings on the APL for their length and use of special panels and details. Standard guardrail, guardrail transitions or other special treatments shall not be included within the limits of the end anchorage assembly. See the manufacturer drawings for the alignment of the end anchorage assemblies with respect to the normal guardrail

5. Flared or parallel end anchorage assemblies shall not be used in medians where horizontal clearance requires the use of a back rail.

6. Each of the various end anchorage assemblies have unique features. Careful attention shall be given to the types and orientation of the posts and other components. Refer to the manufacturer's drawings on the APL for the specific

7. For galvanizing requirements of the metallic components see Standard

8. Test Level 3 End Anchorage Assemblies are suitable for all design speeds. However, use a  $53'-1\frac{1}{2}''$  long TL-3 End Anchorage Assembly shown on the QPL for Design Speeds greater than or equal to 50 mph and a 40'-7 $\frac{1}{2}$ " long TL-2 End Anchorage Assembly Shown on the APL for Design Speeds less than or equal to 45

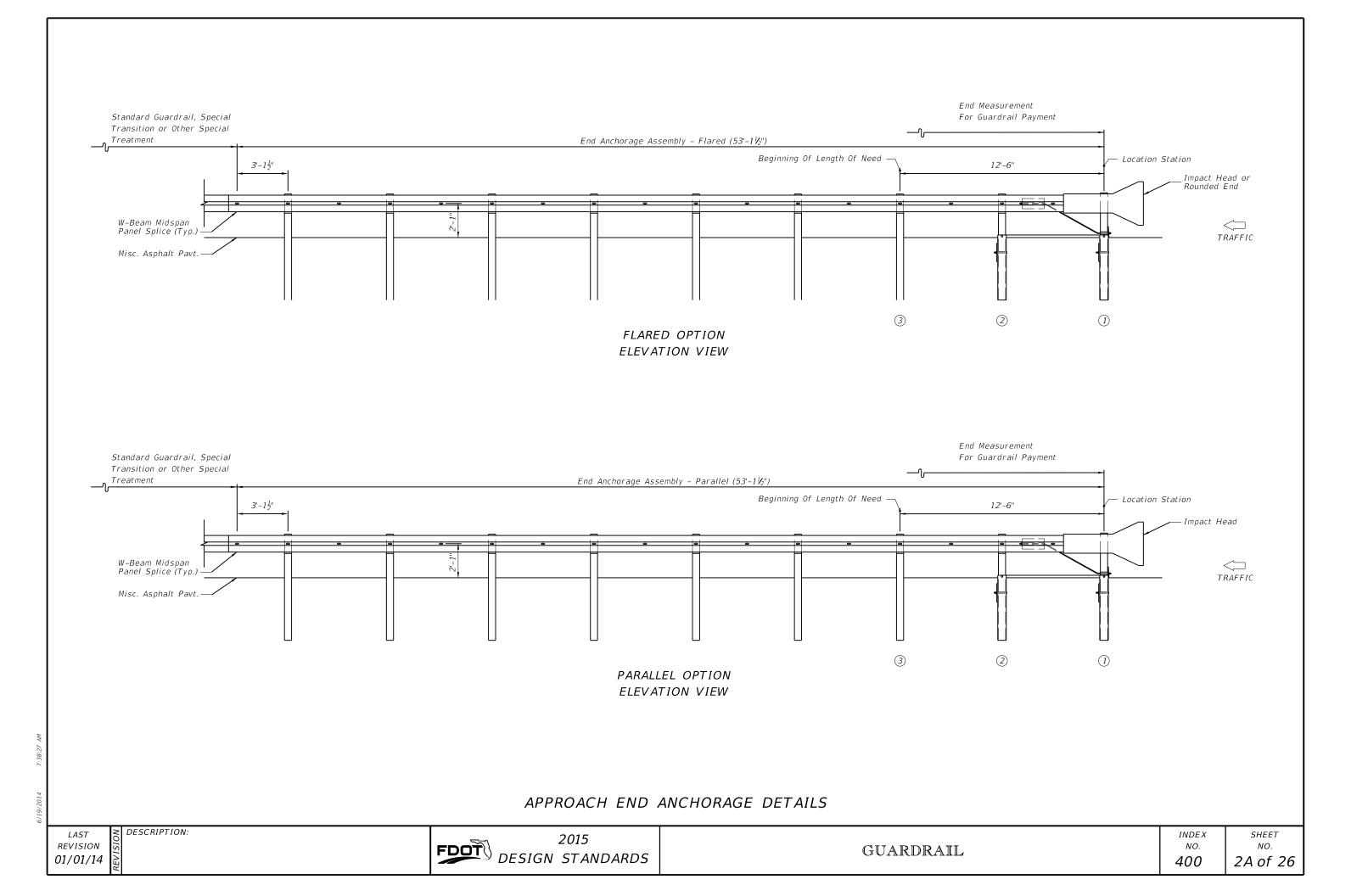
9. Flared end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly - Flared, EA.

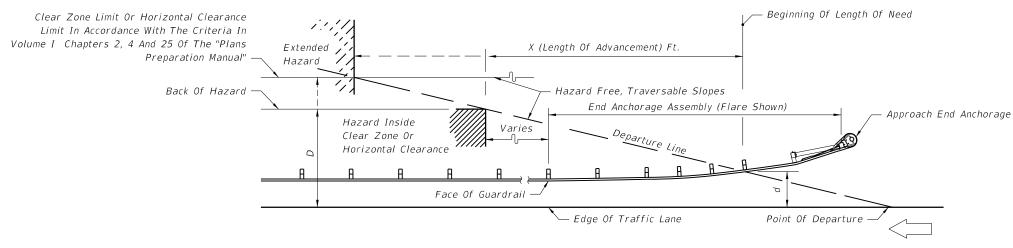
Parallel end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly - Parallel, EA.

The unit price for end anchorage assemblies shall be full compensation for furnishing and installing all components in accordance with these drawings, the plans and the manufacturer's detailed drawings, procedures and specifications.

# APPROACH END ANCHORAGE DETAILS

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Design Speed mph	X (Length Of Advancement) Ft. (See NOTES 1 & 2)
≤ 45	= 16 (D-d)
≥ 50	= 13 (D-d)

### NOTES

- 1. Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.
- 2. The flared end anchorage with 4' nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

#### Equation Variables:

D = Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

d = Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

For flared and parallel end anchorage assemblies the beginning length of need is to be set at the center of post #3. That is, the departure line must intersect the face of the rail at post #3.

For flared end anchorage assemblies the offset distance "d" will equal the normal guardrail offset measured from the face of the guardrail to the edge of the near approach travel lane plus 1'-2" for 45 mph or less and 1'-9 $\frac{1}{4}$ " for greater than 45 mph.

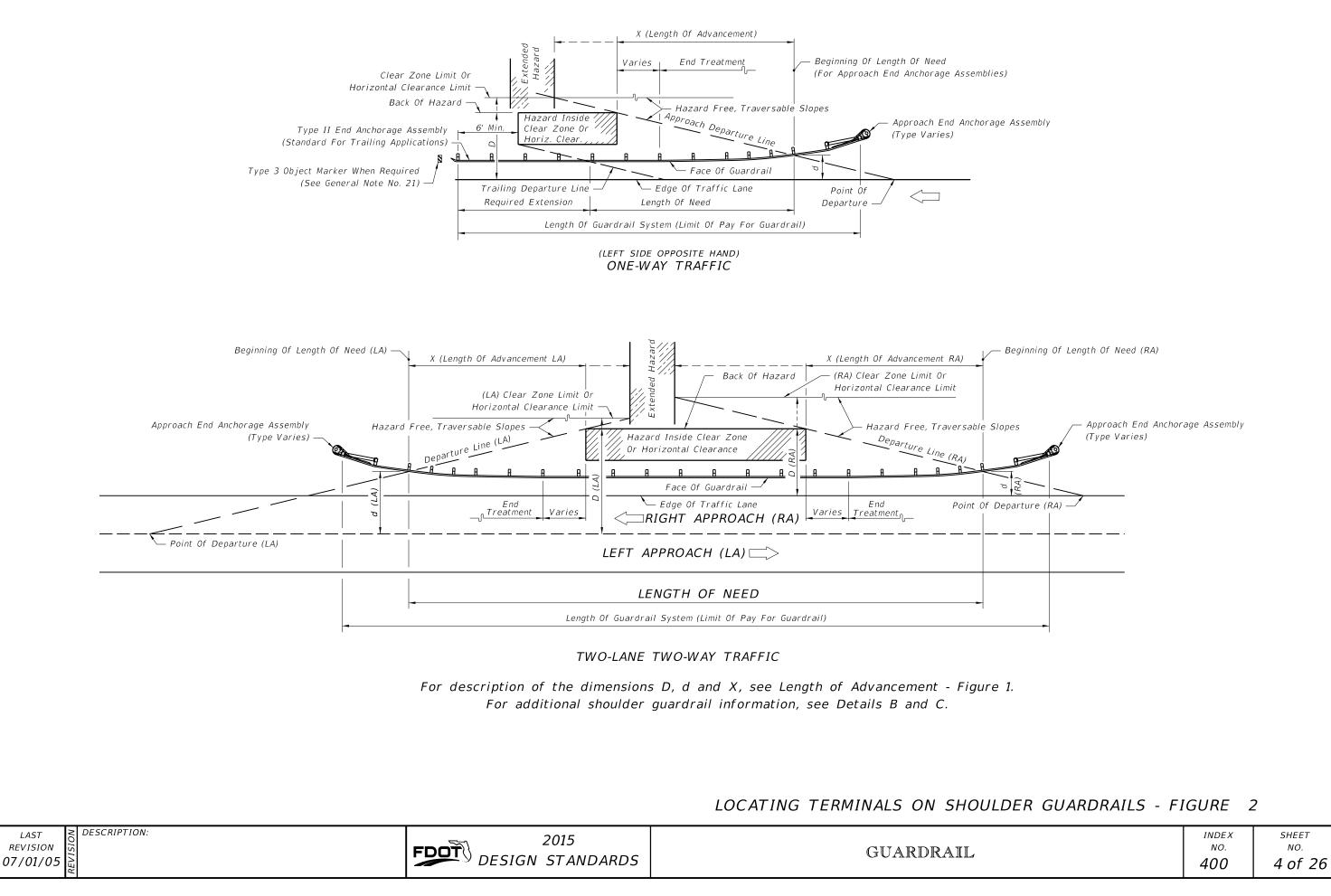
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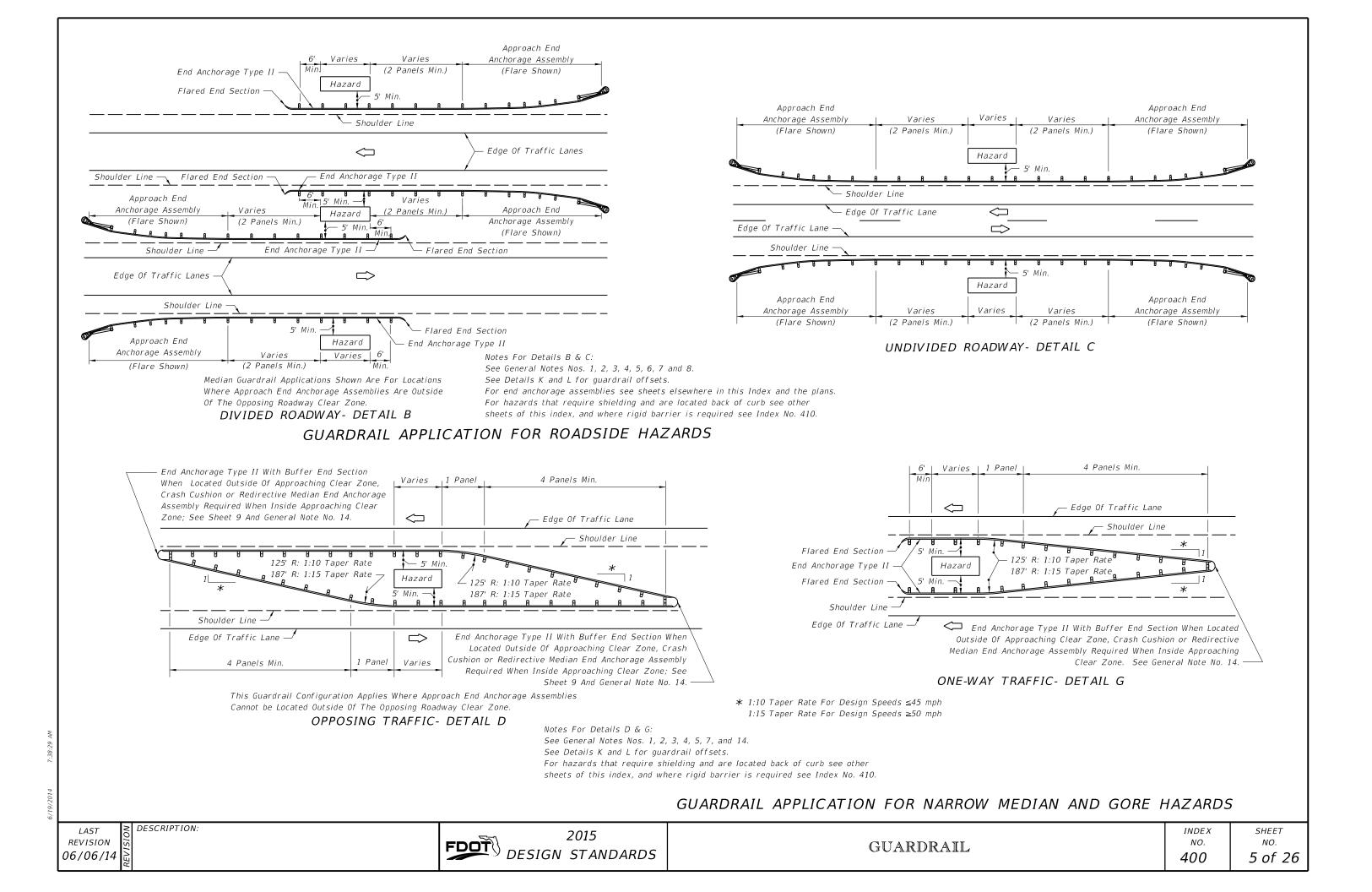
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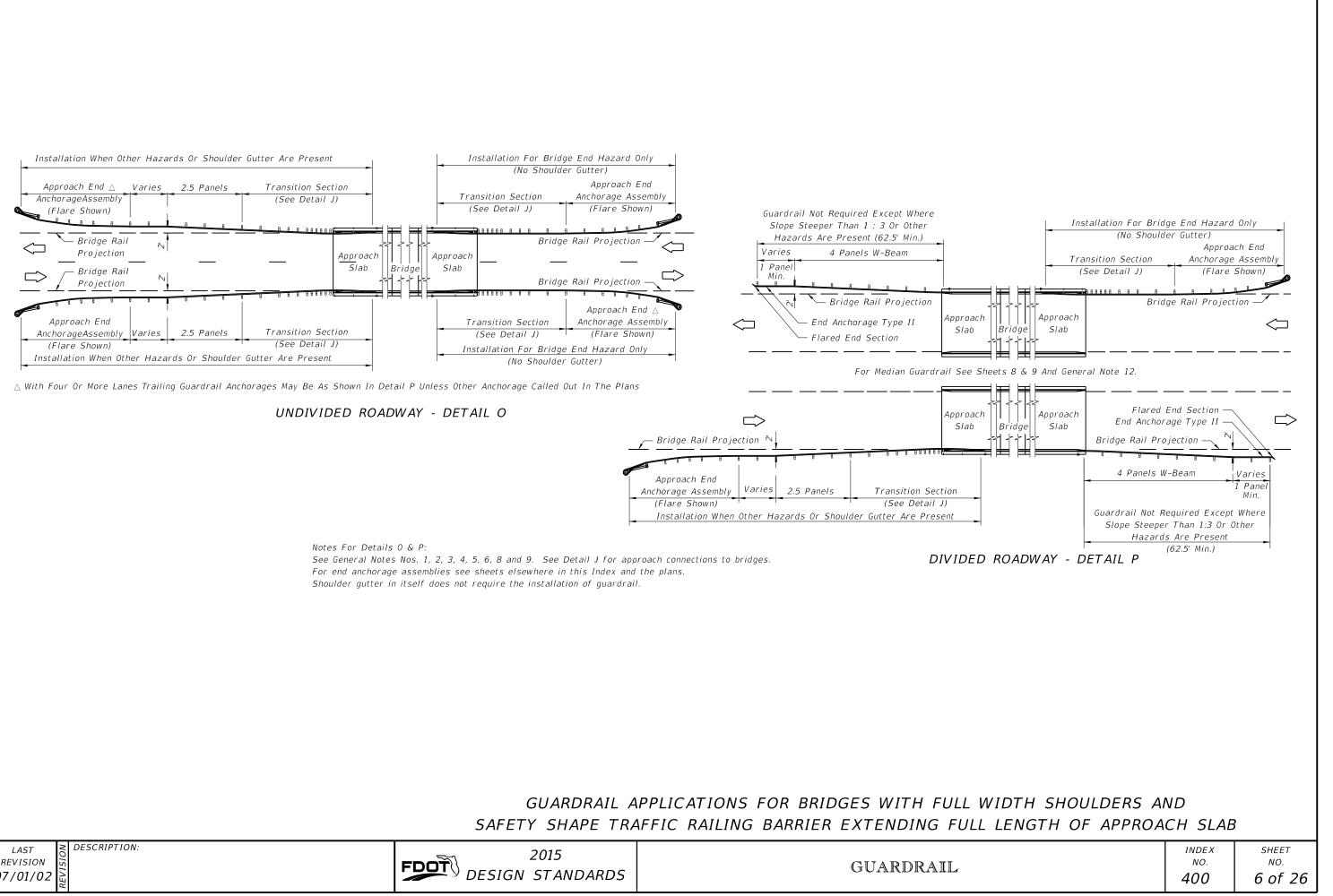
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# LENGTH OF ADVANCEMENT - FIGURE 1

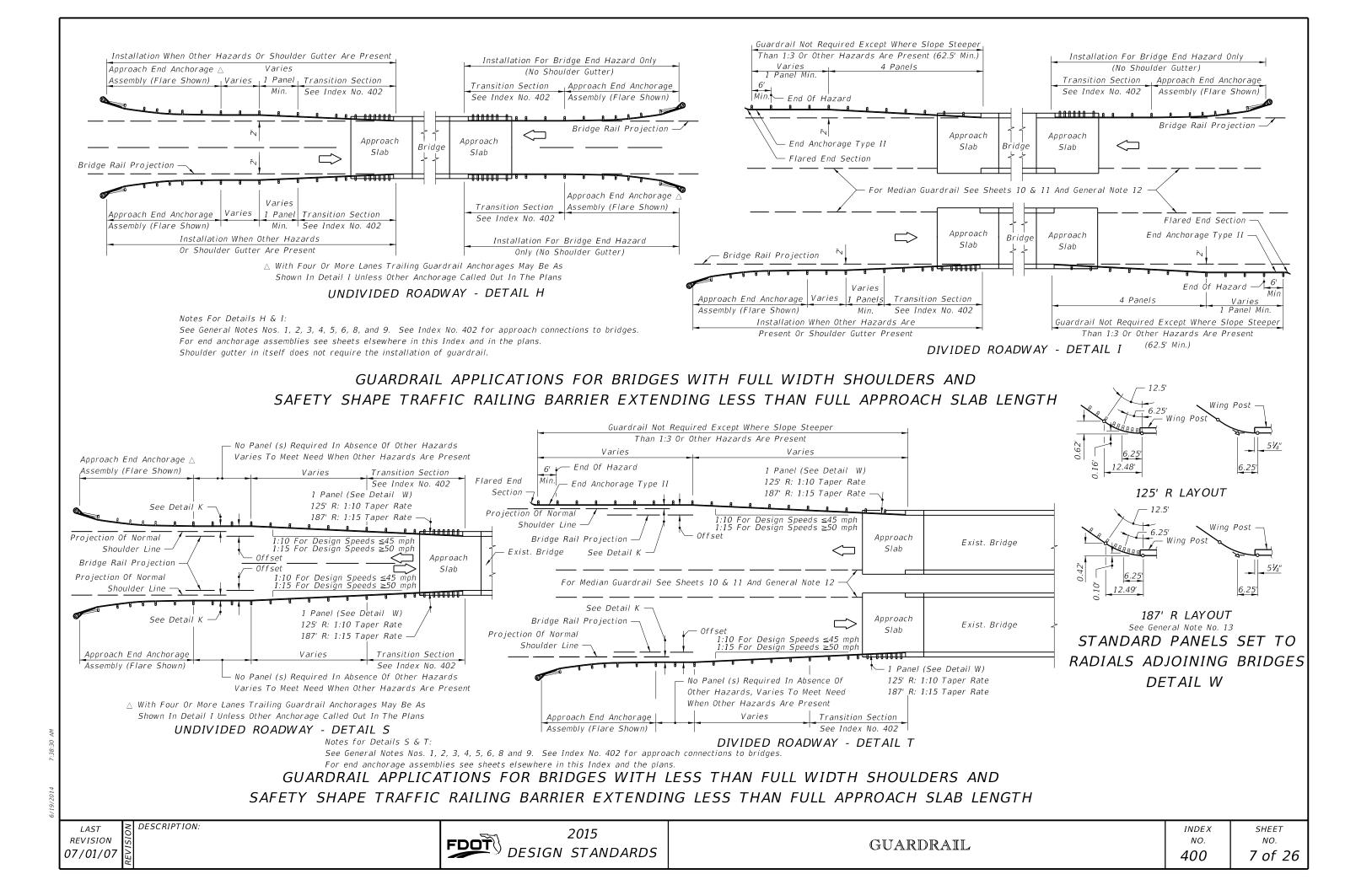
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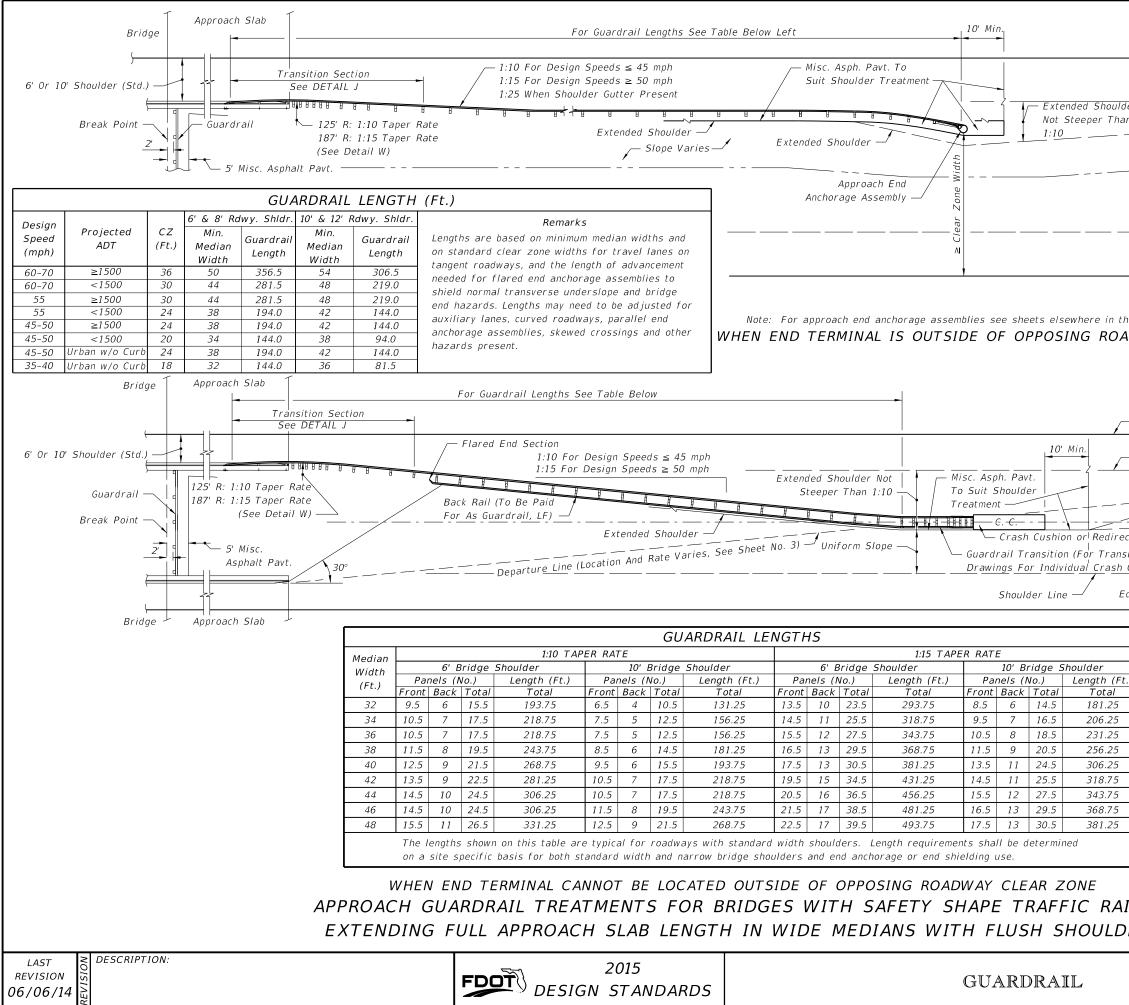




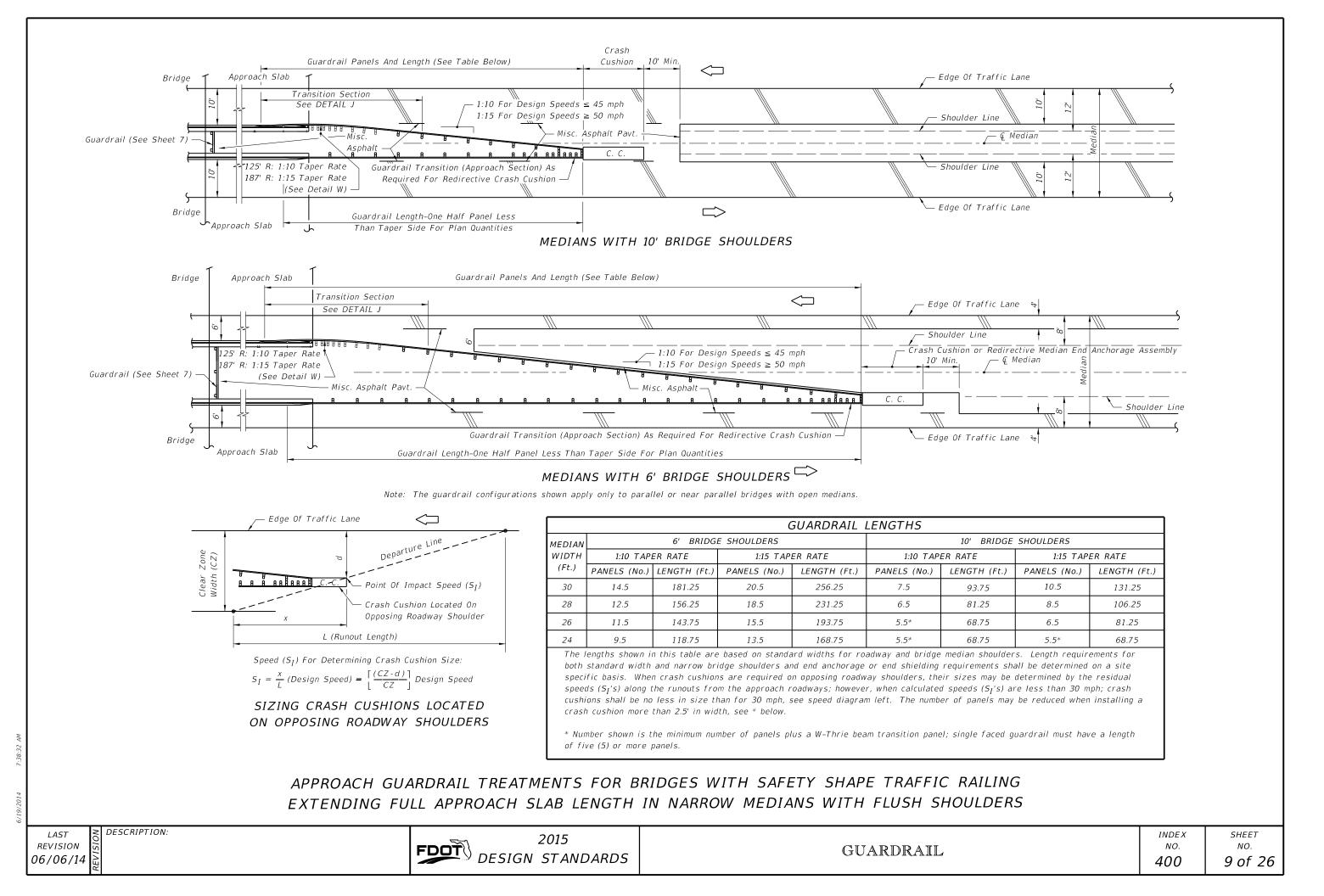


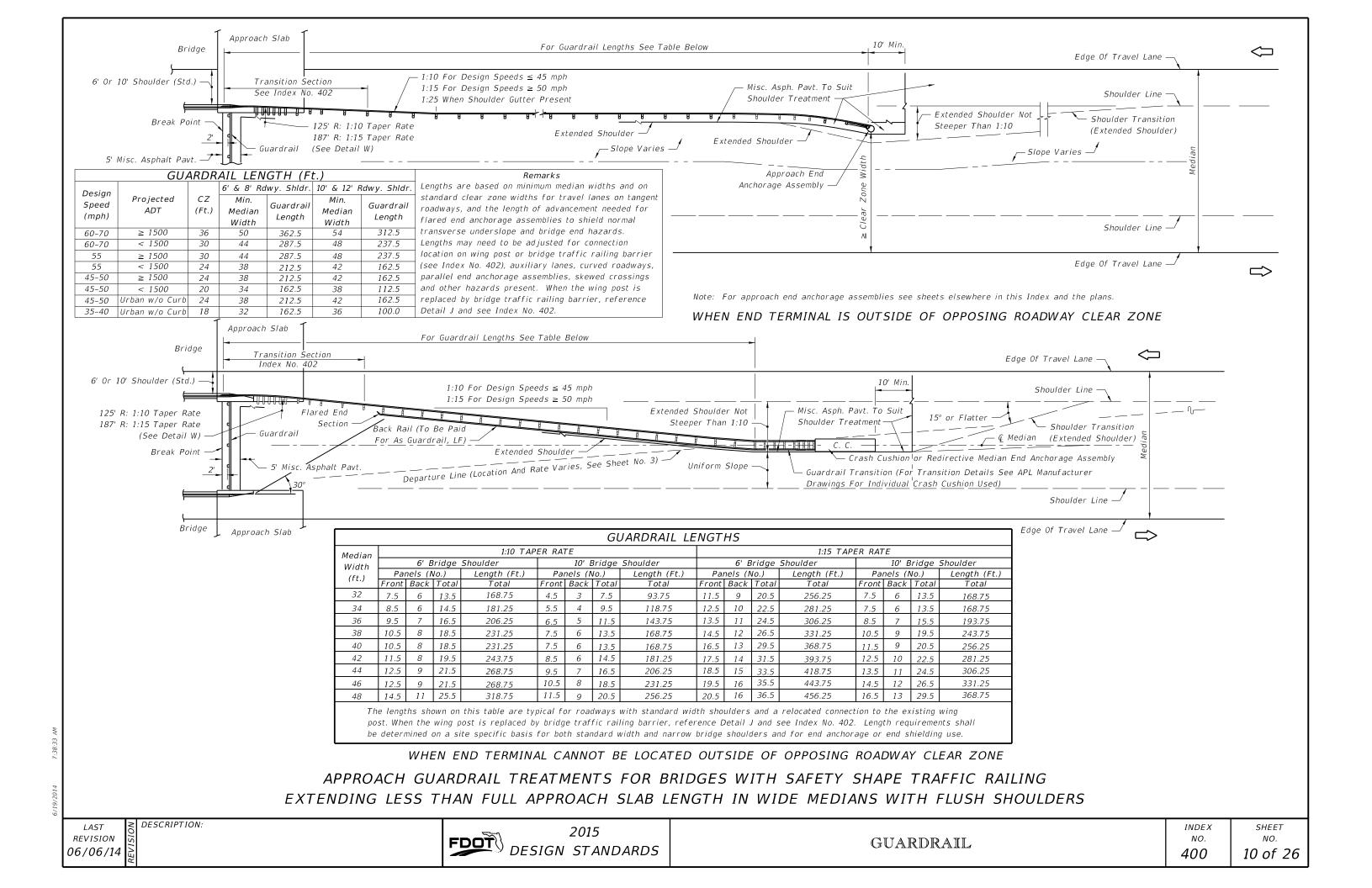
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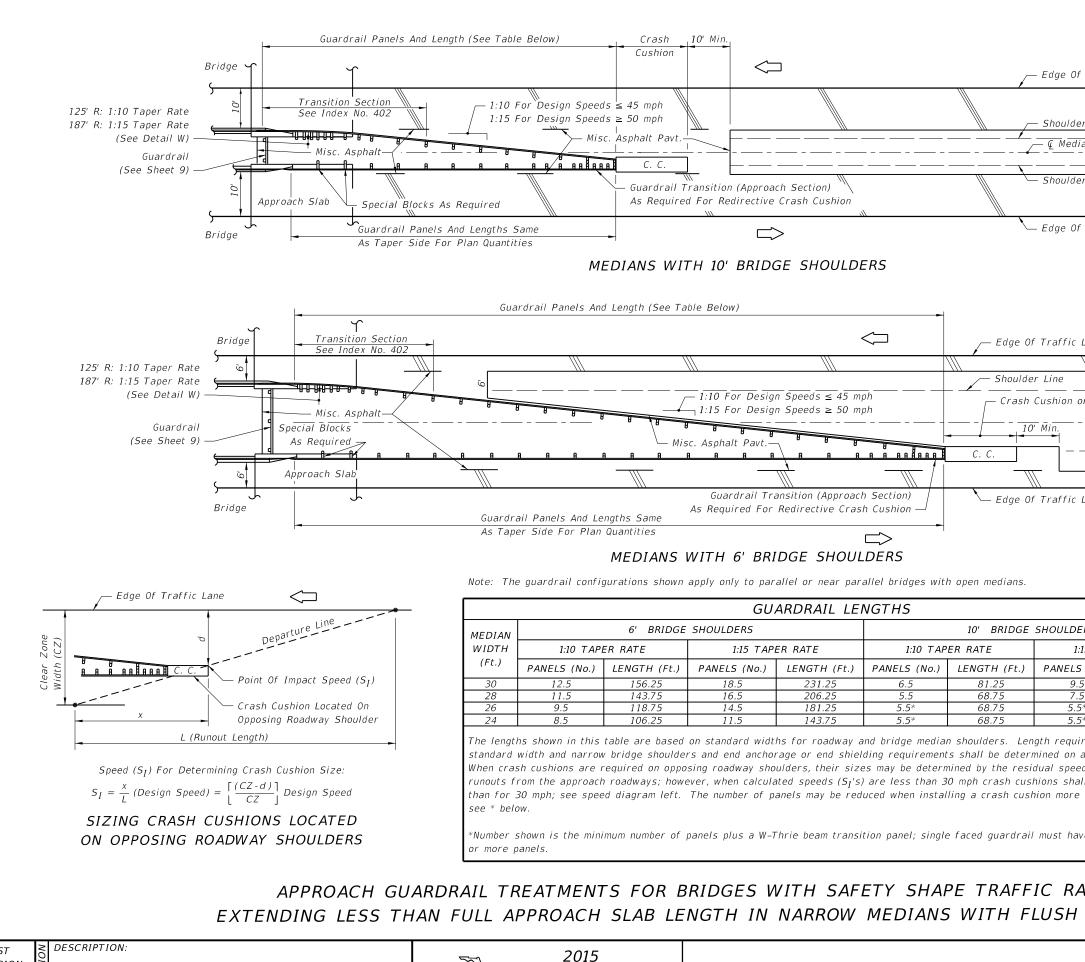




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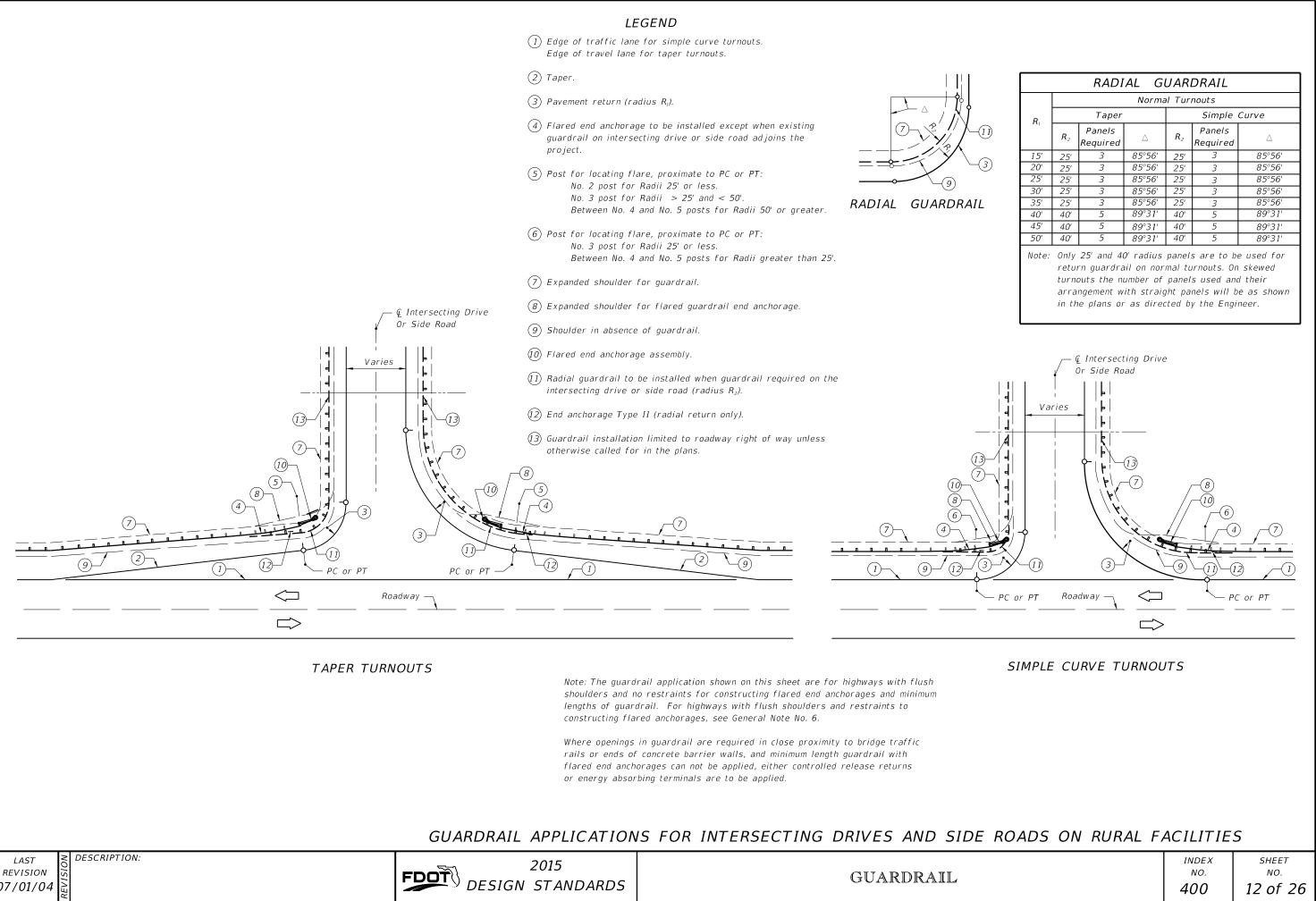
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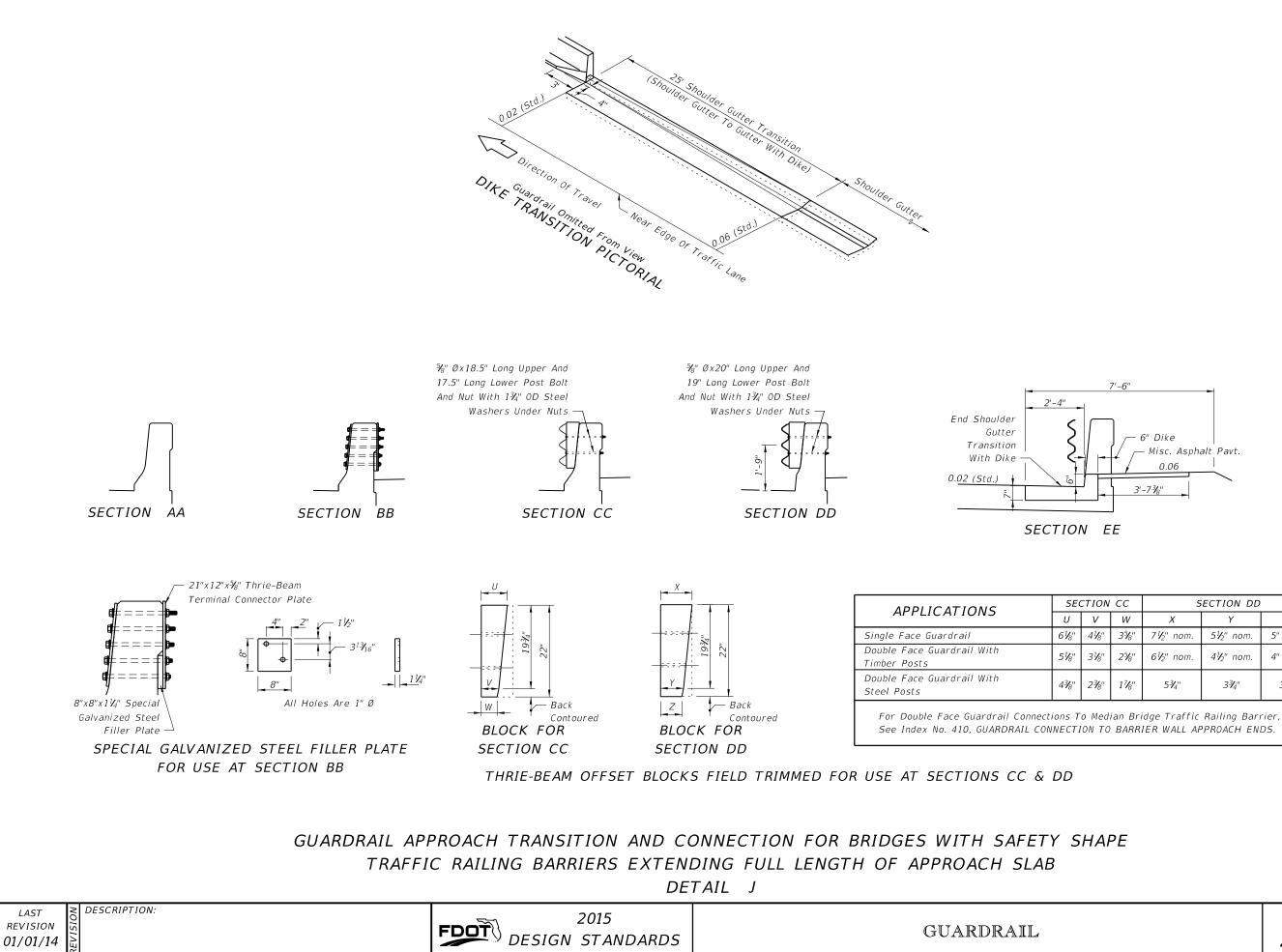


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GUARDRAIL

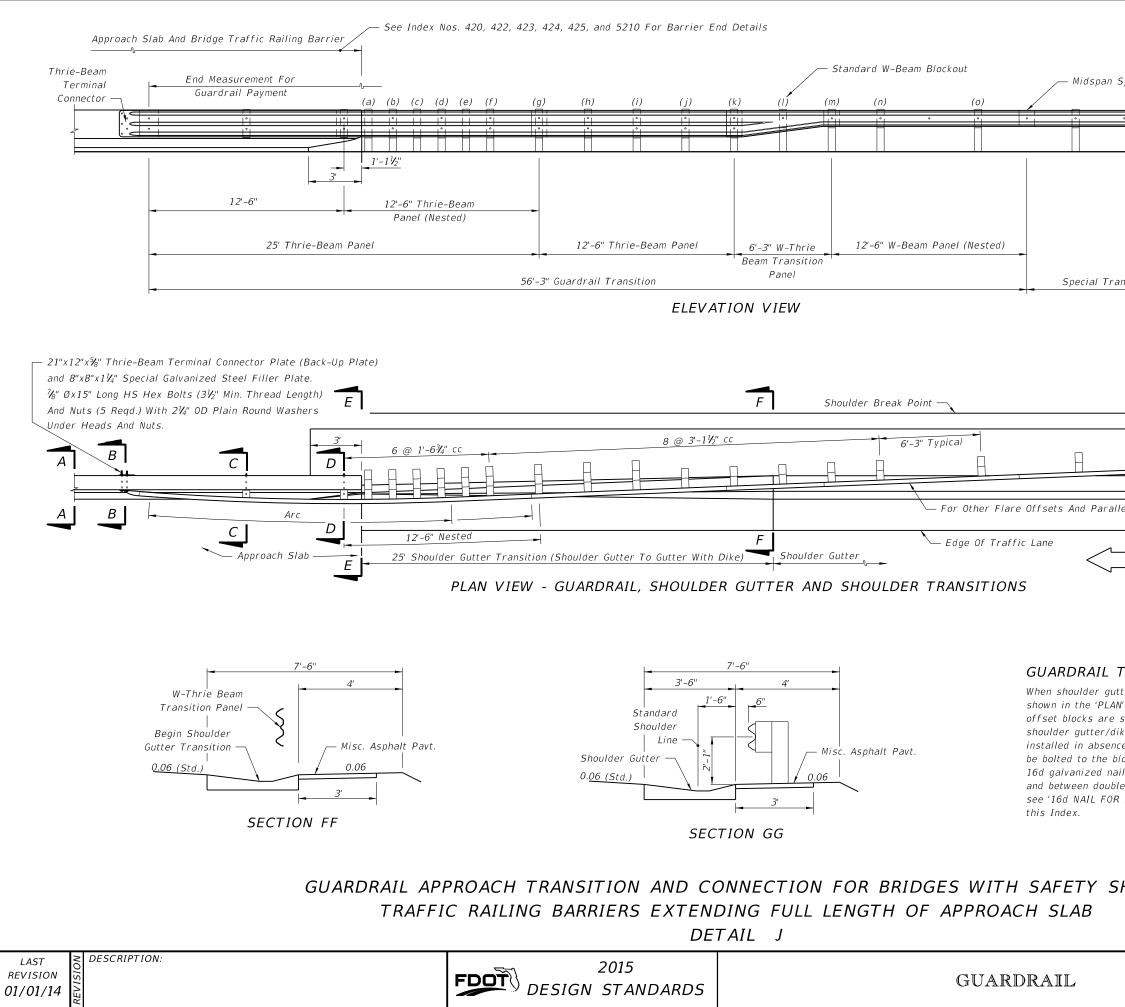


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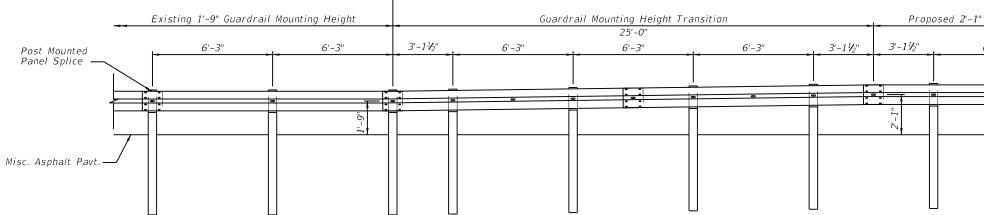
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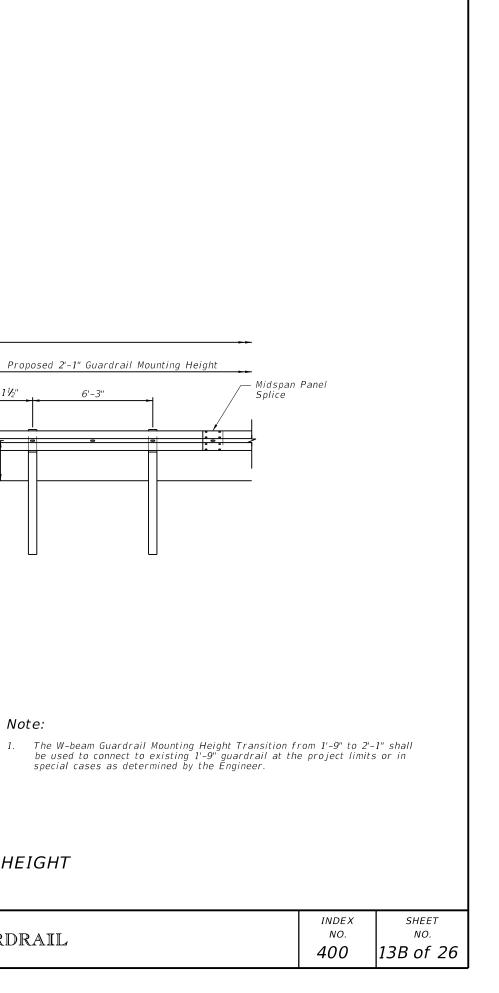
			be used to c special cases
1 107/01/0	TRAN	SITION FROM 1'-9" TO 2'-1" W	-BEAM GUARDRAIL MOUNTING HEIGHT
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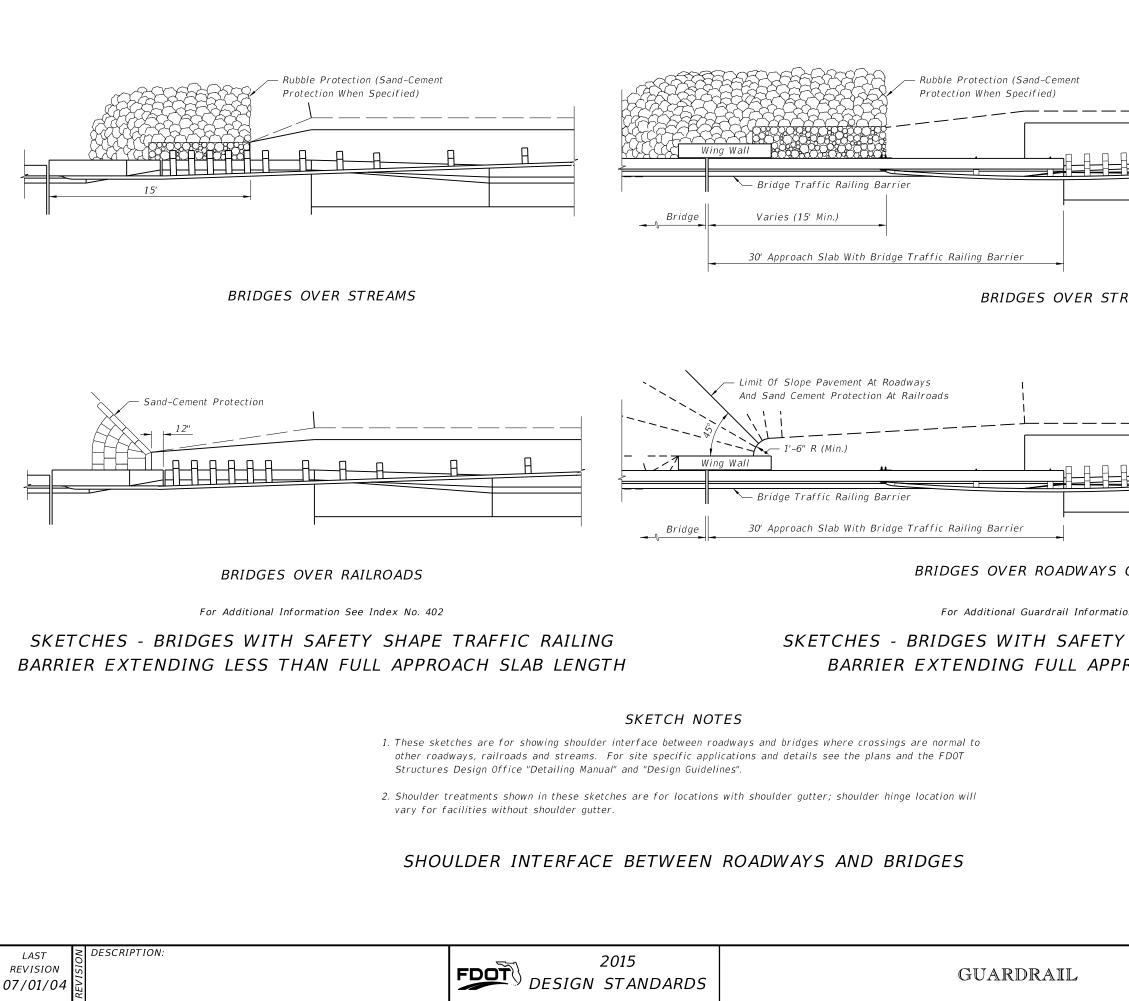


ELEVATION VIEW

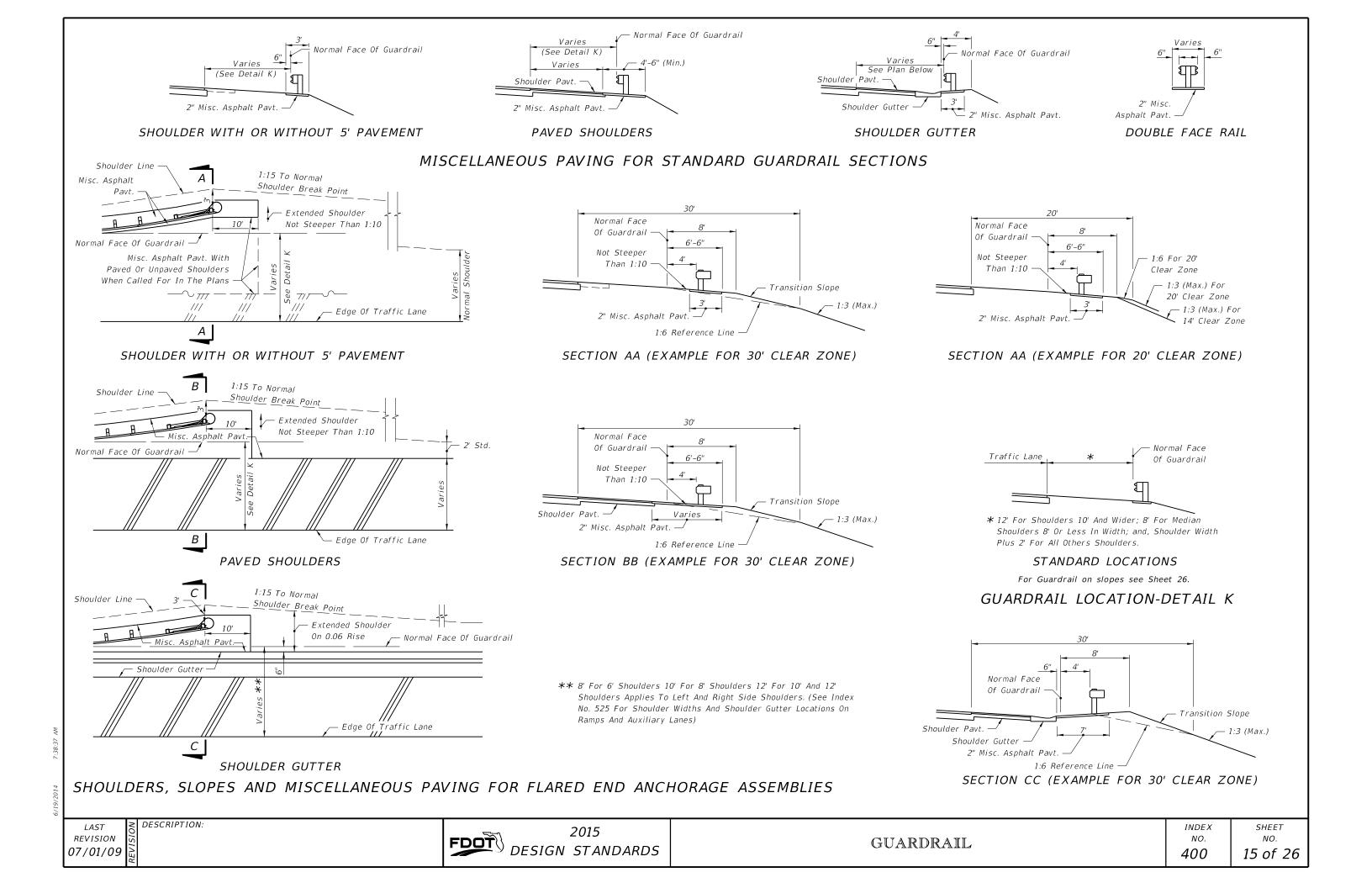
End Measurement For Guardrail Payment

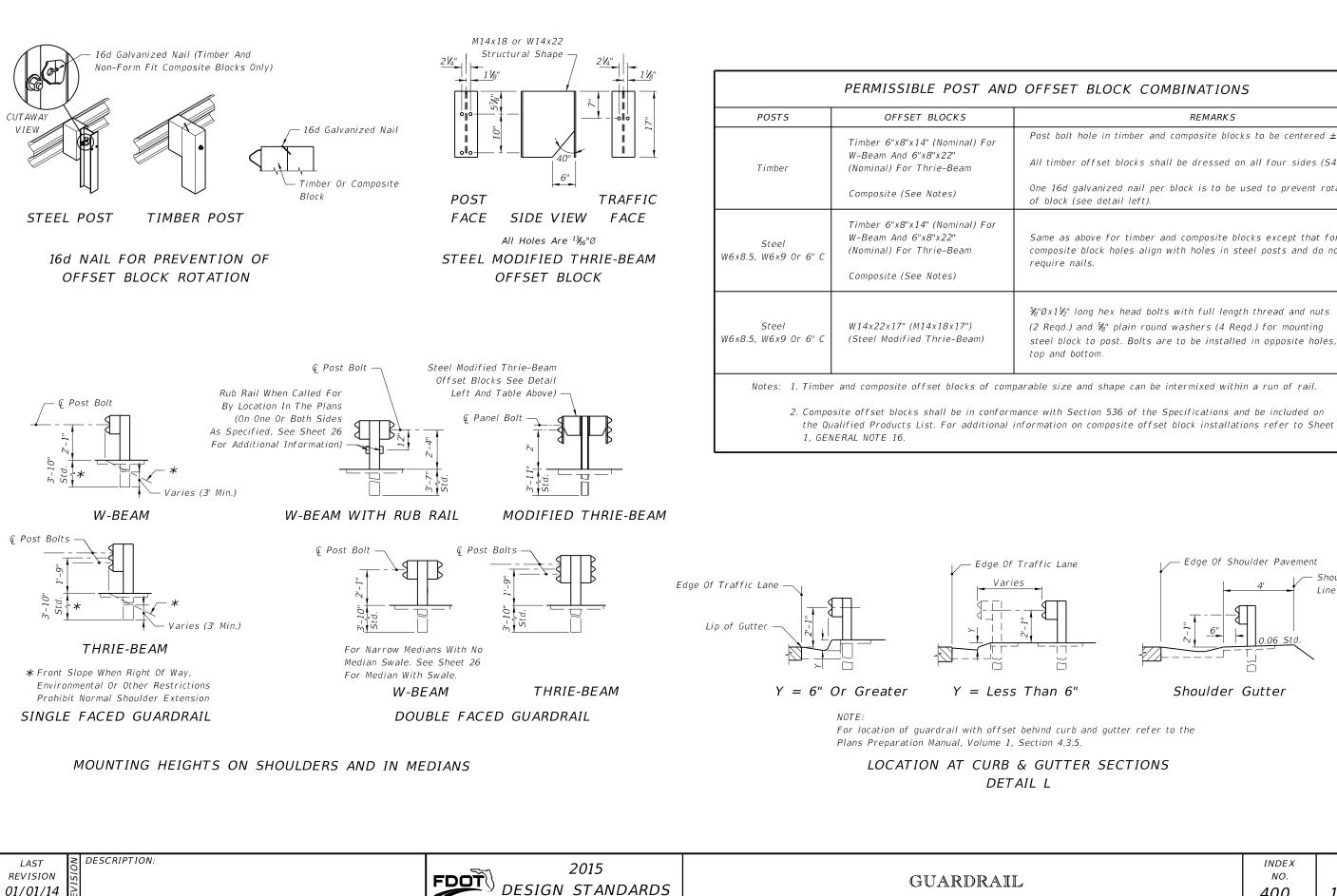
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ΞT	BLOCK	COMBINATIONS

#### REMARKS

Post bolt hole in timber and composite blocks to be centered  $\pm \frac{1}{4}$ ").

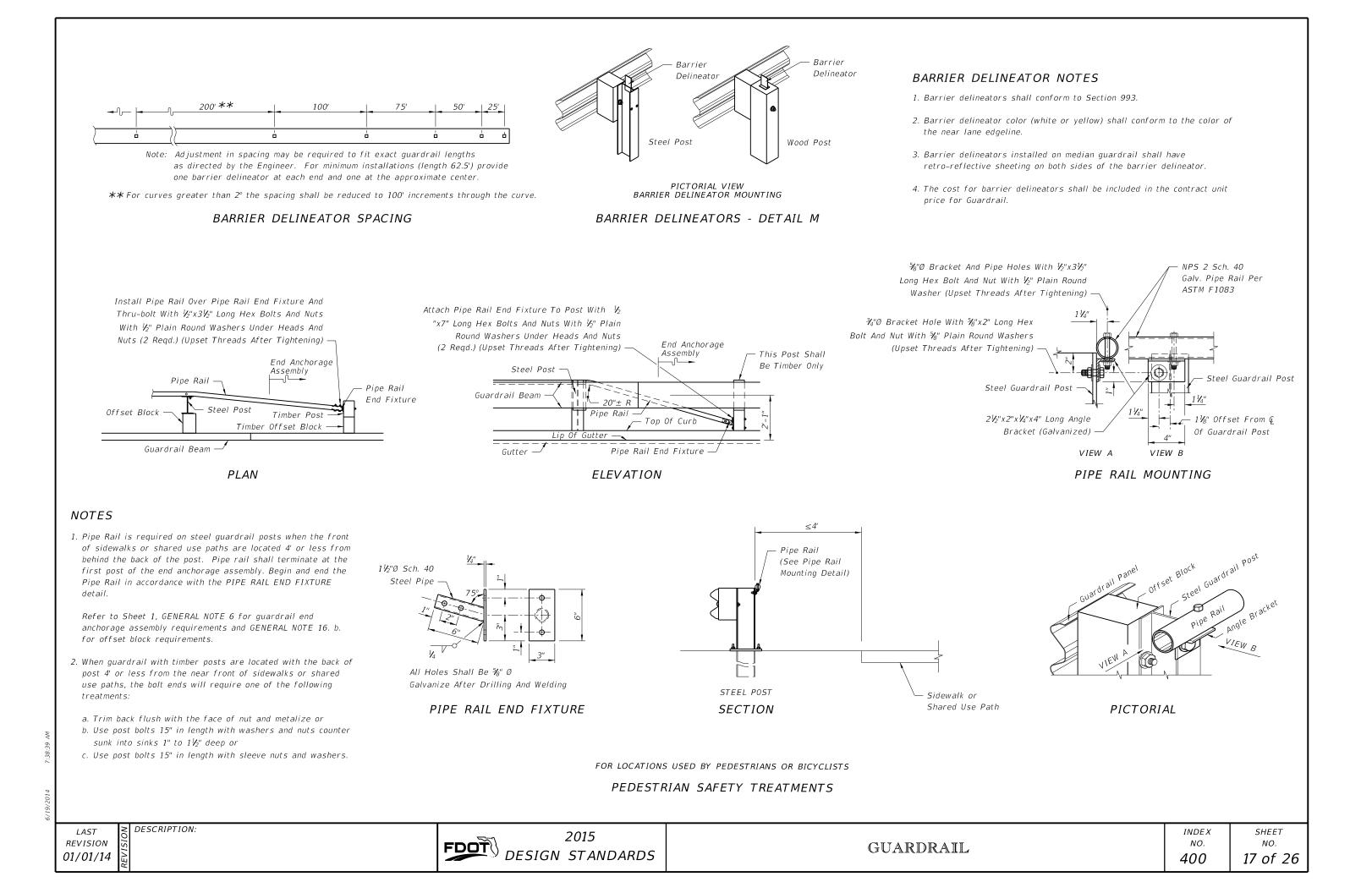
All timber offset blocks shall be dressed on all four sides (S4S).

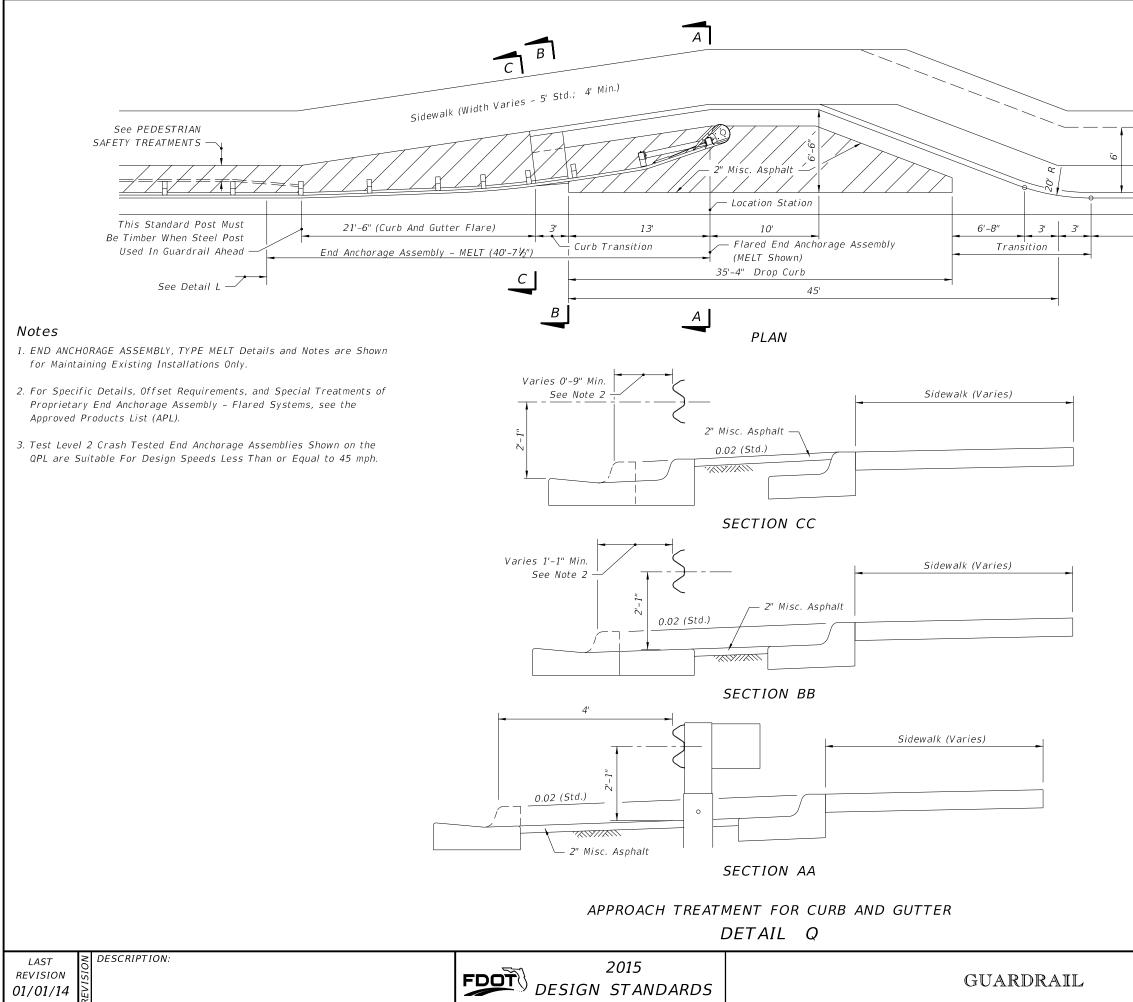
One 16d galvanized nail per block is to be used to prevent rotation of block (see detail left).

Same as above for timber and composite blocks except that form fit composite block holes align with holes in steel posts and do not

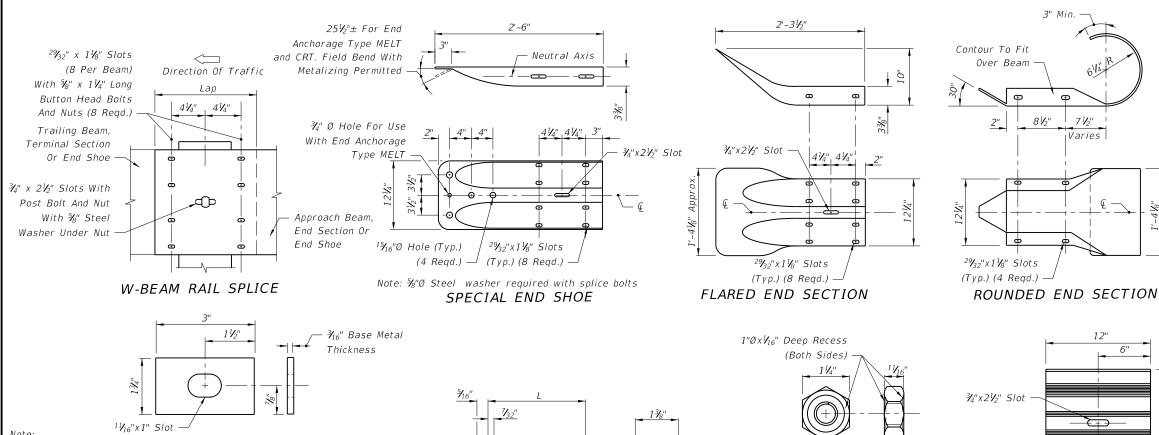
 $\frac{5}{8}$ "Øx1 $\frac{1}{2}$ " long hex head bolts with full length thread and nuts (2 Reqd.) and  $\frac{5}{8}$ " plain round washers (4 Reqd.) for mounting steel block to post. Bolts are to be installed in opposite holes,

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	4'	Line
6" Shoulder	Gutter	
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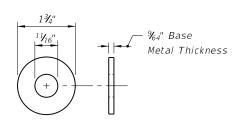
Sidewalk With	out Utility St	rip	
10'	Curb And Gut	ter Type F	
$\langle \square$			
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	I	INDEX	SHEET
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		400	10 01 20



#### Note:

For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary to accomplish alignment or where the posts bolt head shows tendency to pull through the rail slot. Washers installed on guardrail, between end anchorages, prior to July 1, 1990 may remain in place until the guardrail is relocated or until repairs require removal and reinstallment of a post bolt.

## (RECTANGULAR PLATE WASHER) BEAM WASHER



#### Note:

The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by  $\frac{5}{8}$ " Ø hex bolts and nuts and under hex nut for connecting rub rail to wood and steel posts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

### 5⁄8" STEEL WASHER

(In.)	LENGTH (Min.) (In.)	APPLICATION
1 <sup>1</sup> ⁄⁄ <sub>4</sub> "	Full Length	Rail Splice Bolt
10"	4"	Single Or Double Faced Guardrail Timber Or Composite Offset Post Bolt - Block(s) On Steel Post As An Option, A Single 25"* Long Post Bolt May Be Used
18"	4"	Post Bolt – Single Faced Guardrail Timber Posts
25"*	4"	Post Bolt - Double Faced Guardrail Timber Posts Double Faced Guardrail Steel Posts
	al bolts having ss than 4".	g lengths of 10" or greater shall have a thread length c

For applications where special bolts having lengths greater than 25" are required, the Contractor may use a 5/8"Ø threaded rod (field cut to length). A hex nut and beam washer shall be used at the guardrail face with no more than  $\mathscr{Y}_4$ " of the threaded rod projecting beyond the top of the nut. The projecting thread on both ends shall be distorted to secure the nuts, and both ends of the threaded rod metalized with organic zinc-rich coating.

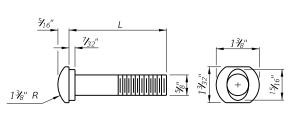
st Use of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced guardrail that results in the bolt projecting more than  $\frac{3}{4}$ " beyond the face of the nut after pull-up shall be trimmed to  $\mathscr{X}_4$ " reveal and metalized with organic zinc-rich coating.

#### $\frac{5}{8}$ " OVAL SHOULDER BUTTON HEAD BOLT

FOOT DESIGN STANDARDS

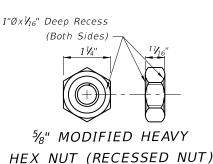
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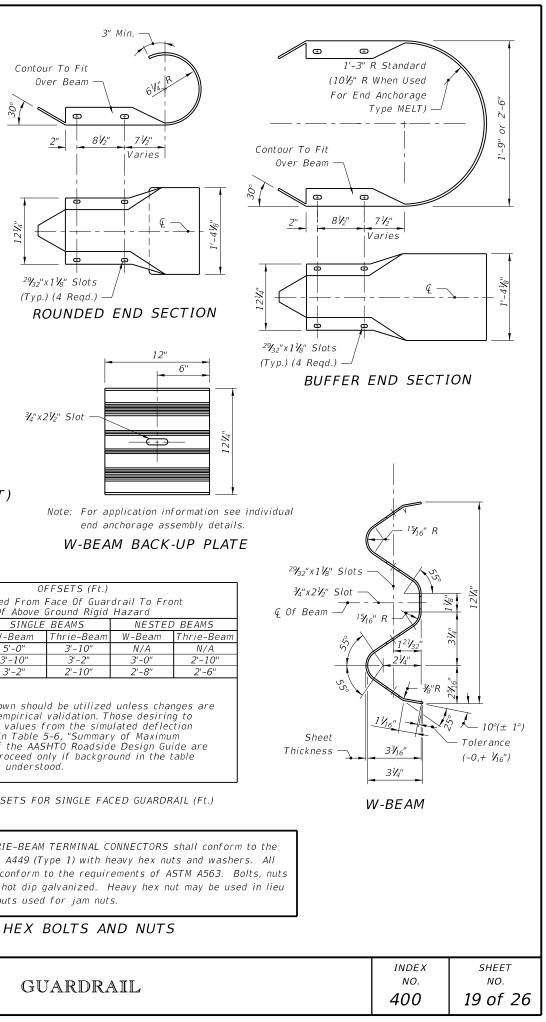
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THREAD

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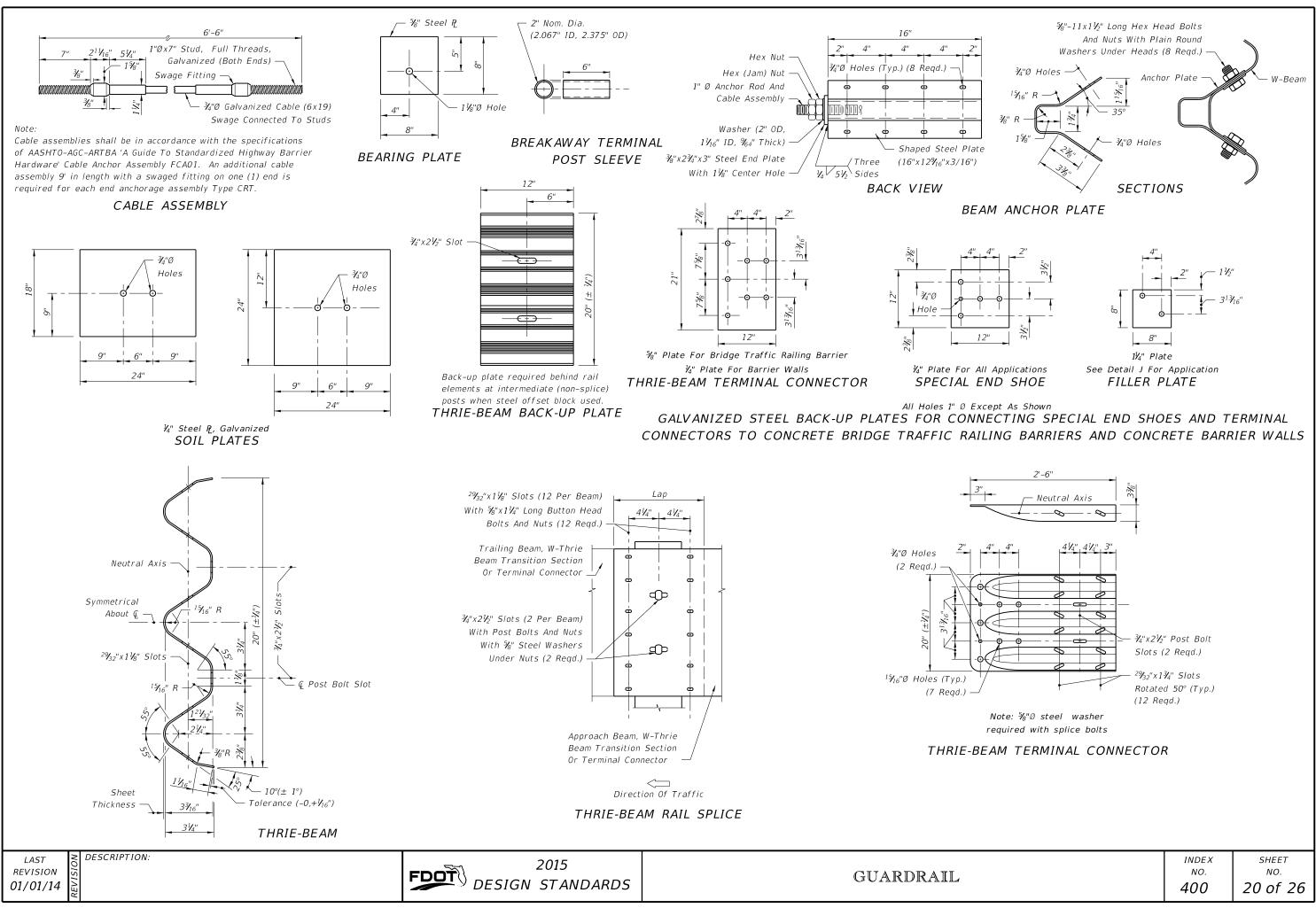
OFFSETS (Ft.)				
Measured From Face Of Guardrail To Front Of Above Ground Rigid Hazard				
POST	SINGLE	NESTEL	) BI	
SPACING (Ft.)	W-Beam	Thrie-Beam	W-Beam	Τh
6'-3"	5'-0''	3'-10"	N/A	
3'-1½"	3'-10"	3'-2"	3'-0"	
1'-6¾"	3'-2"	2'-10"	2'-8"	
Nata				

Note:

The values shown should be utilized unless changes are supported by empirical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5-6, "Summary of Maximum Delfections" of the AASHTO Roadside Design Guide are cautioned to proceed only if background in the table development is understood.

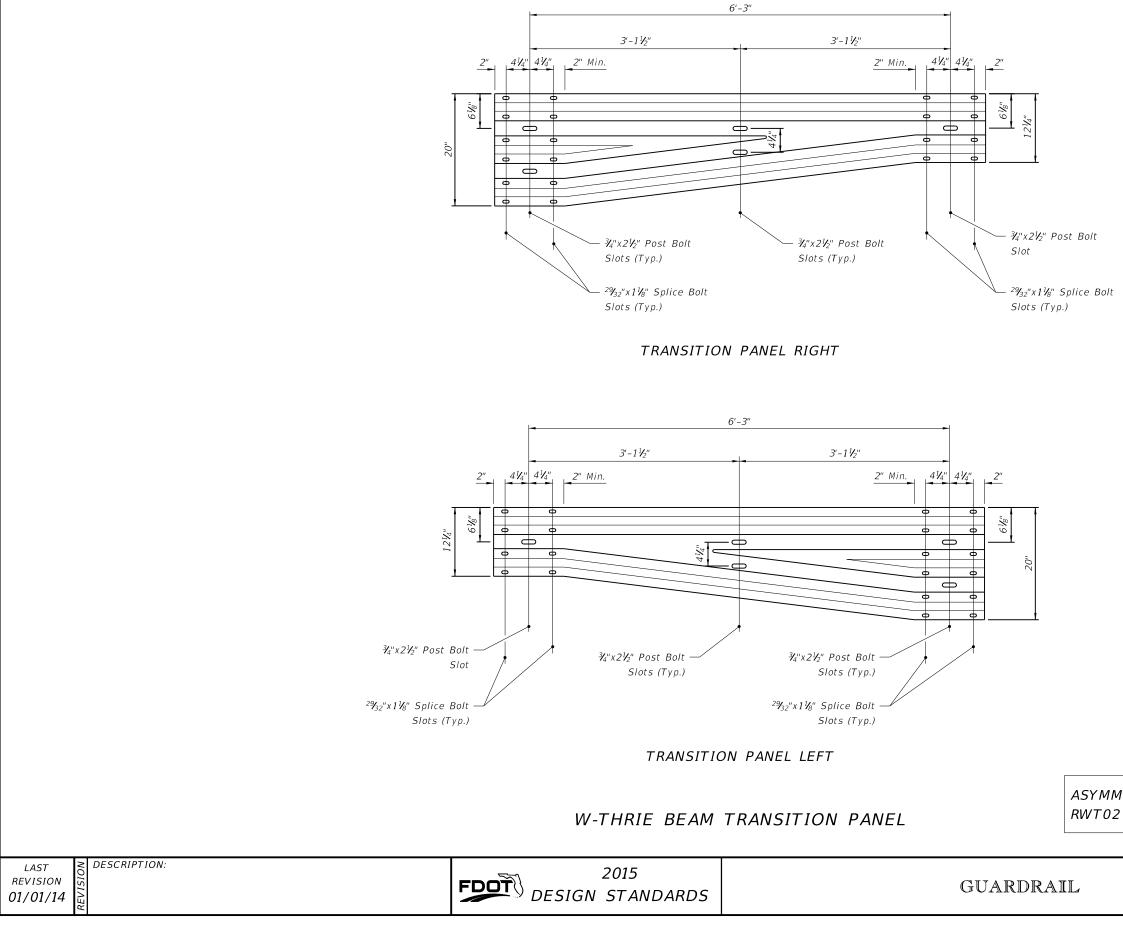
MINIMUM OFFSETS FOR SINGLE FACED GUARDRAIL (Ft.)

HS Hex bolts for THRIE-BEAM TERMINAL CONNECTORS shall conform to the requirements of ASTM A449 (Type 1) with heavy hex nuts and washers. All other hex bolts shall conform to the requirements of ASTM A563. Bolts, nuts and washers shall be hot dip galvanized. Heavy hex nut may be used in lieu of hex nuts and hex nuts used for jam nuts.

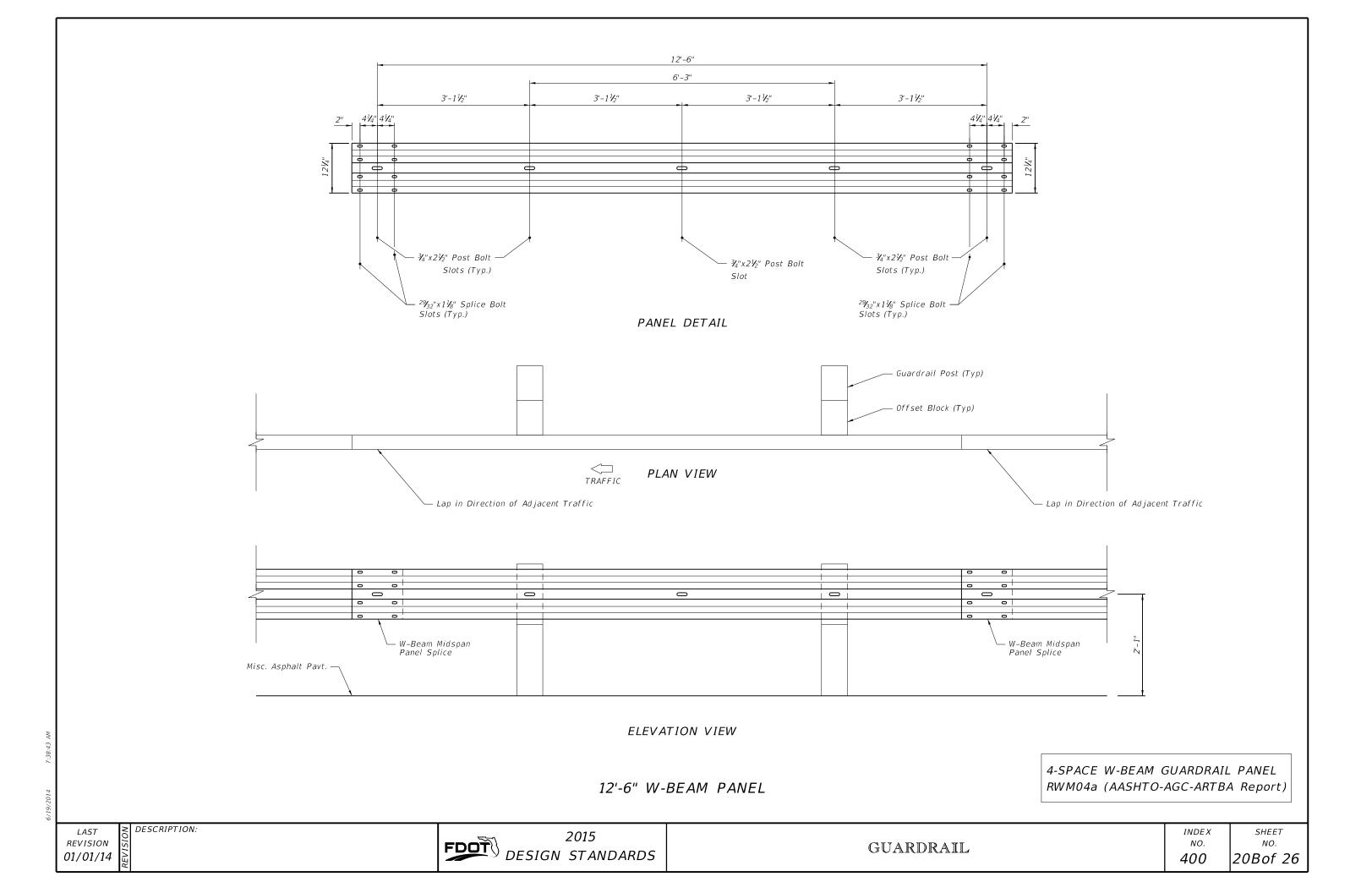


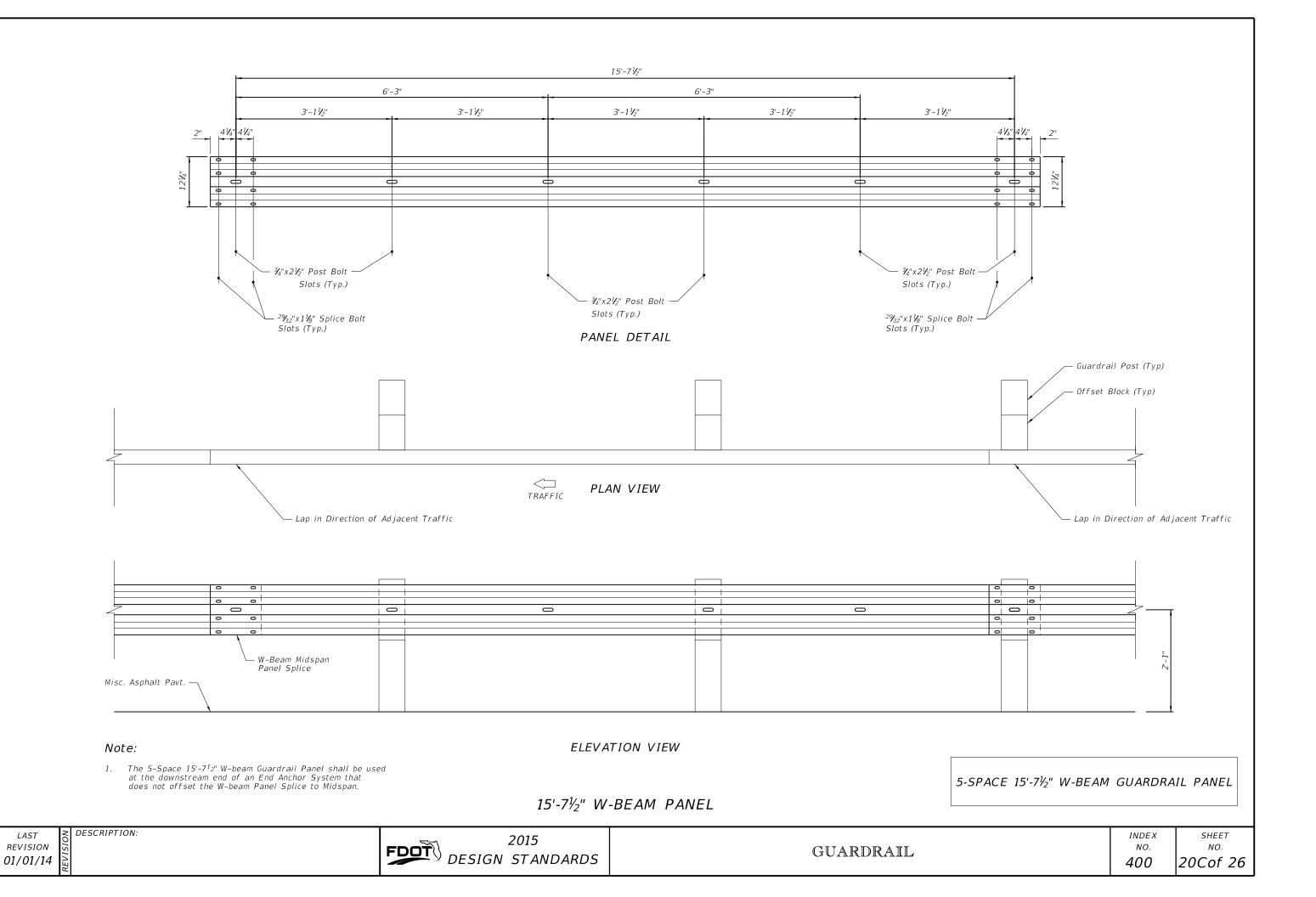
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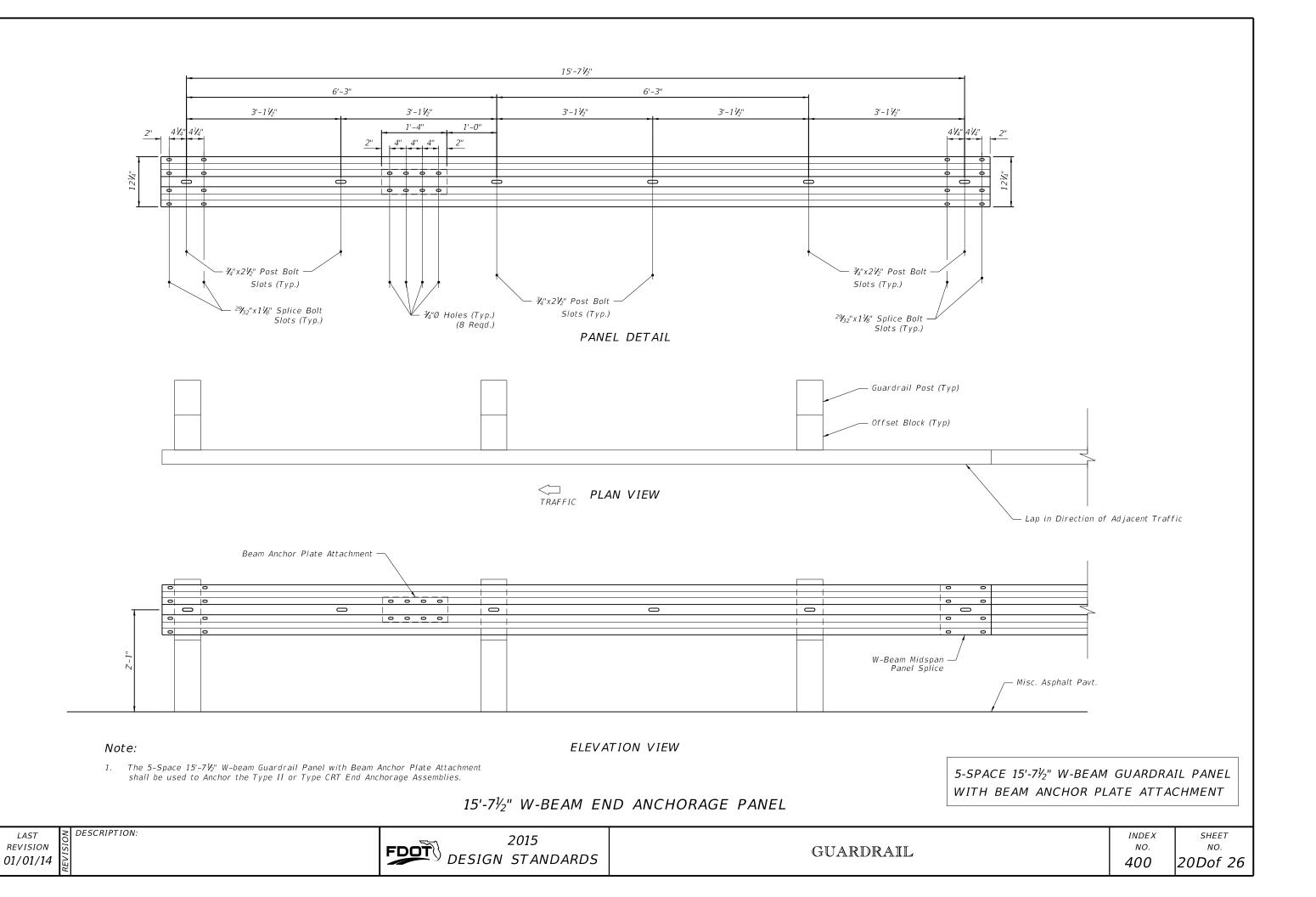
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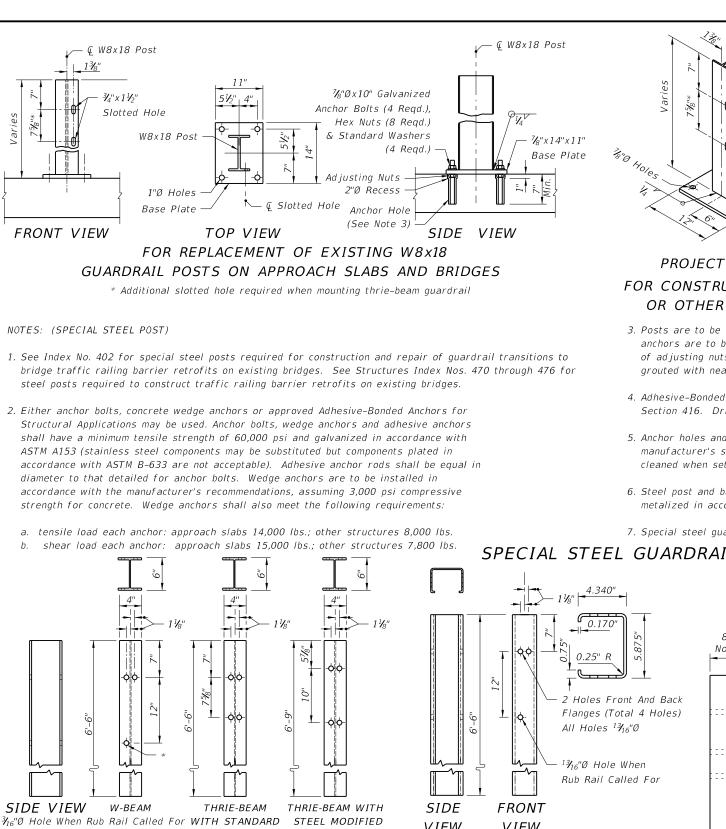


METRICAL W-THRIE BEAM TRANSITION PANEL 2 (AASHTO-AGC-ARTBA Report) 10 Gauge		
	INDEX NO.	SHEET NO.
	400	20Aof 26



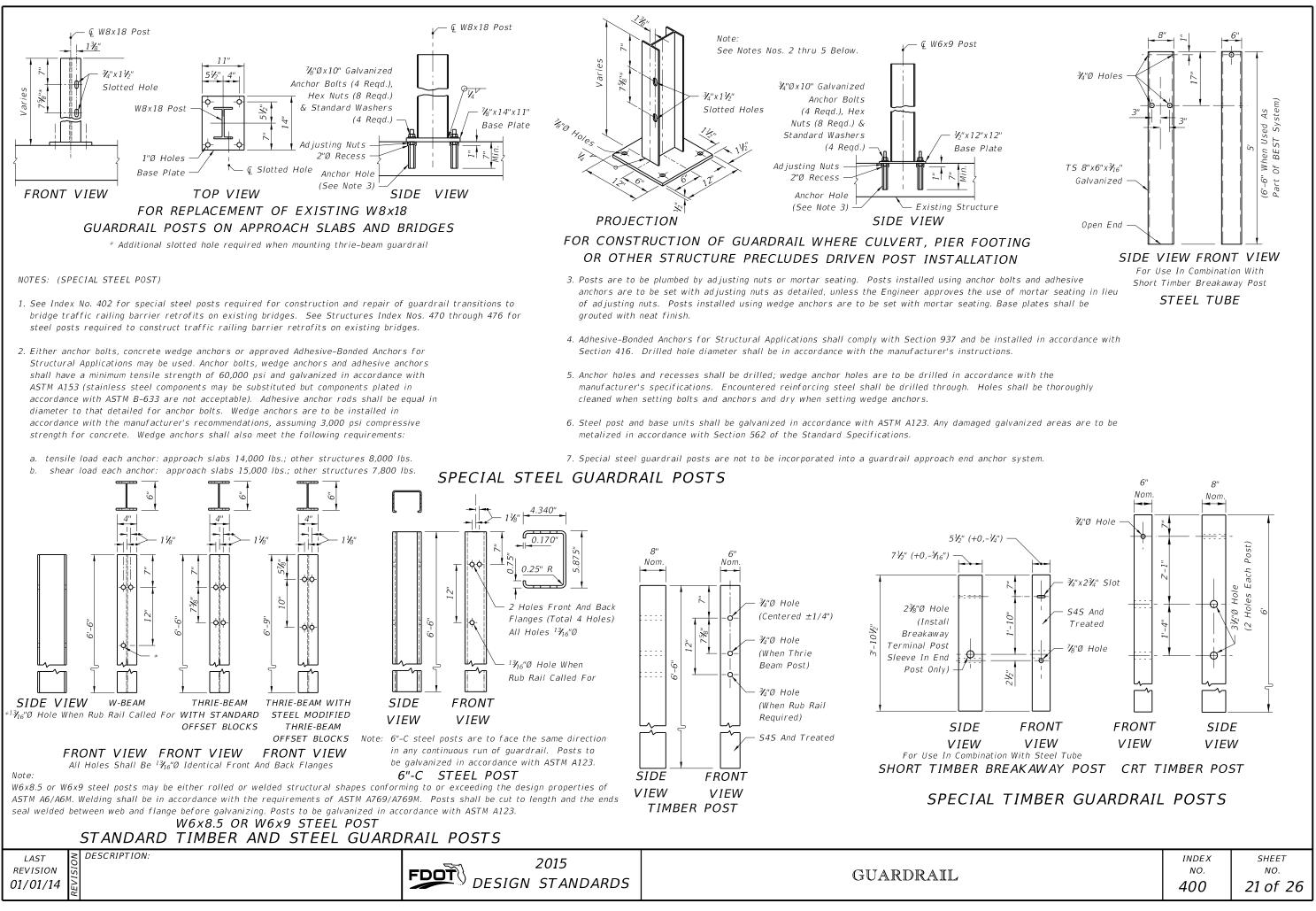


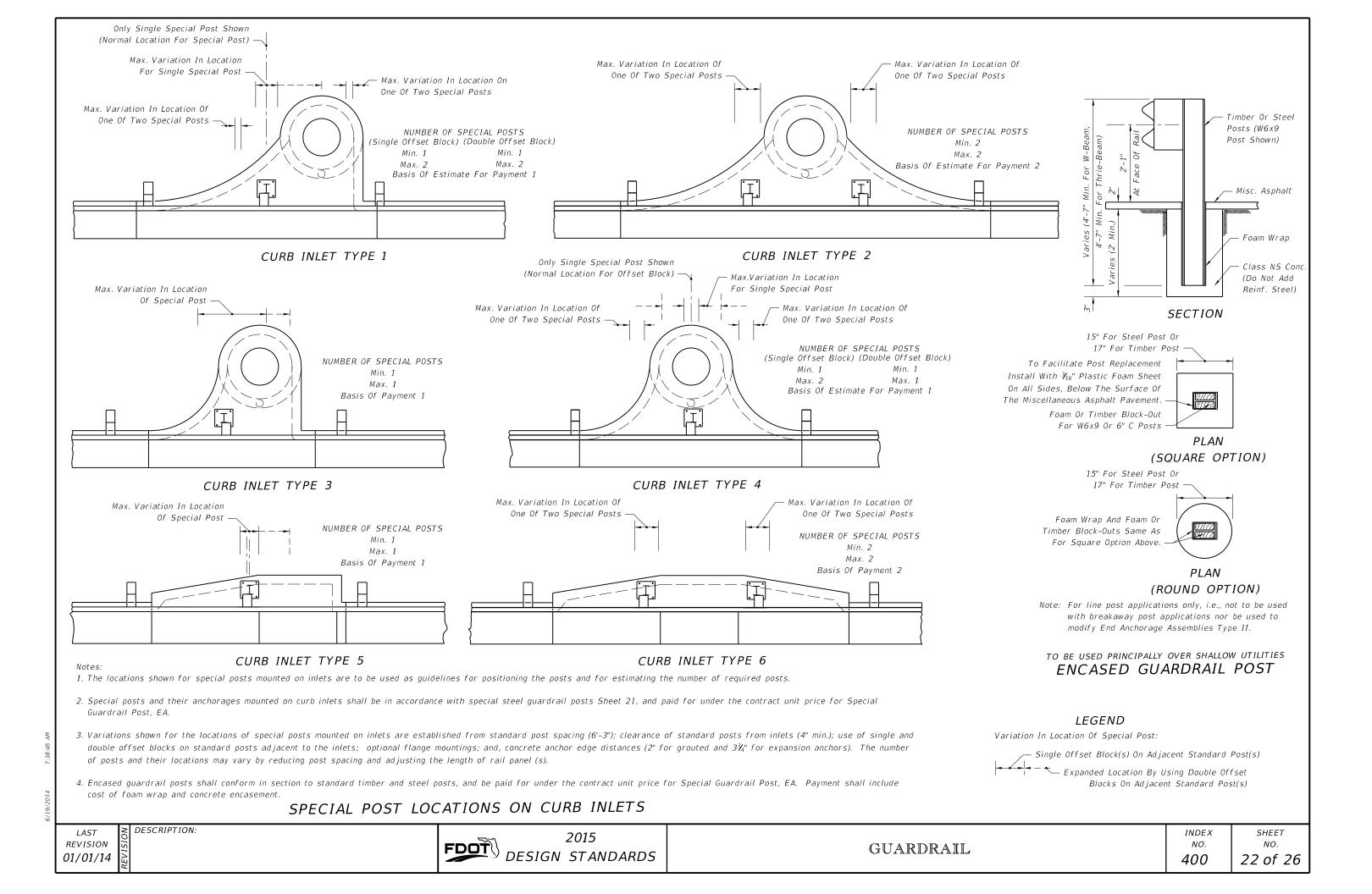


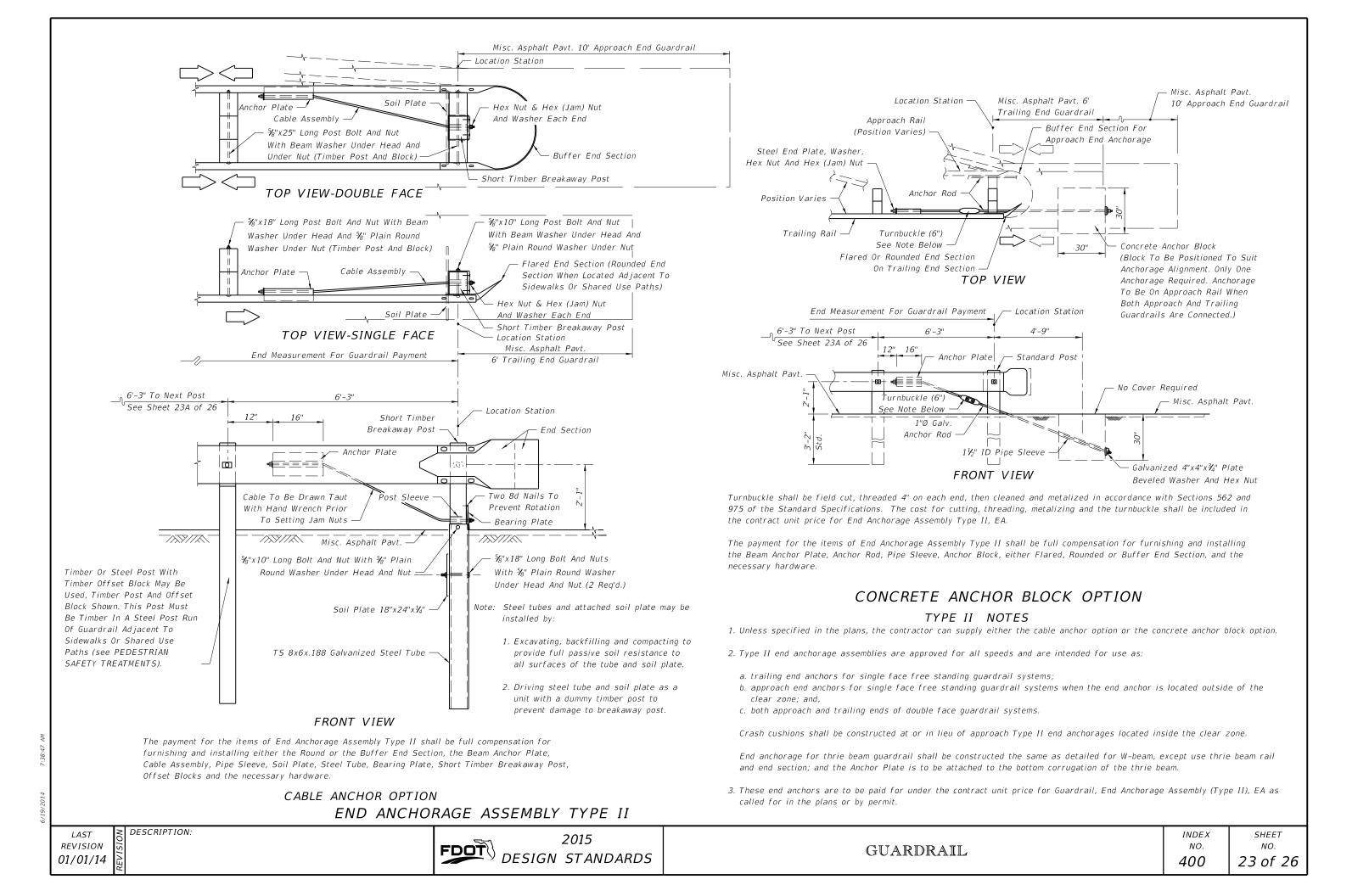


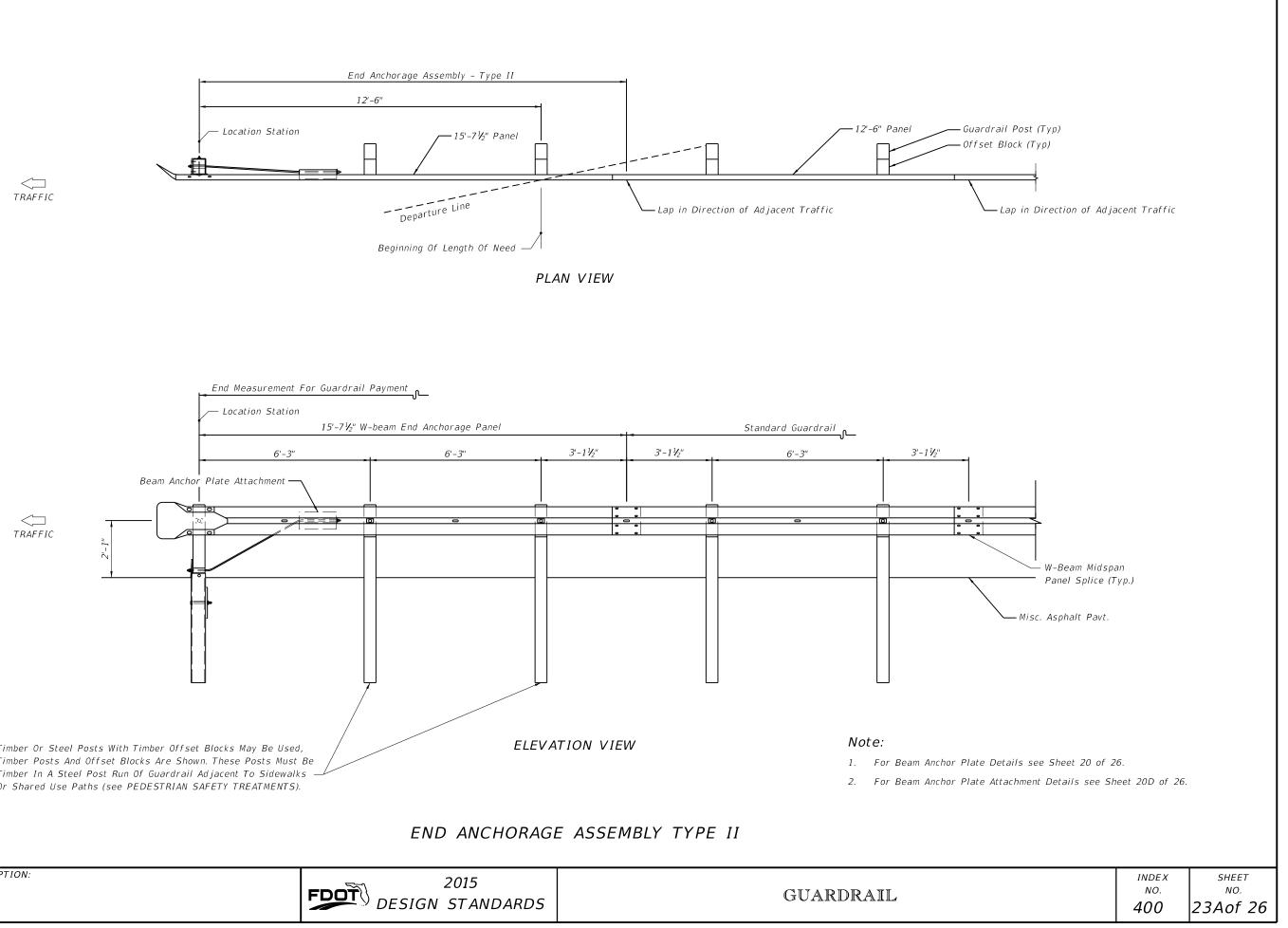
¾"Øx10" Galvanized ¾"×1½" Anchor Bolts Slotted Holes (4 Reqd.), Hex Nuts (8 Reqd.) & Standard Washers (4 Regd.) Adjusting Nuts 2"Ø Recess Anchor Hole (See Note 3) Existing Structure PROJECTION SIDE VIEW

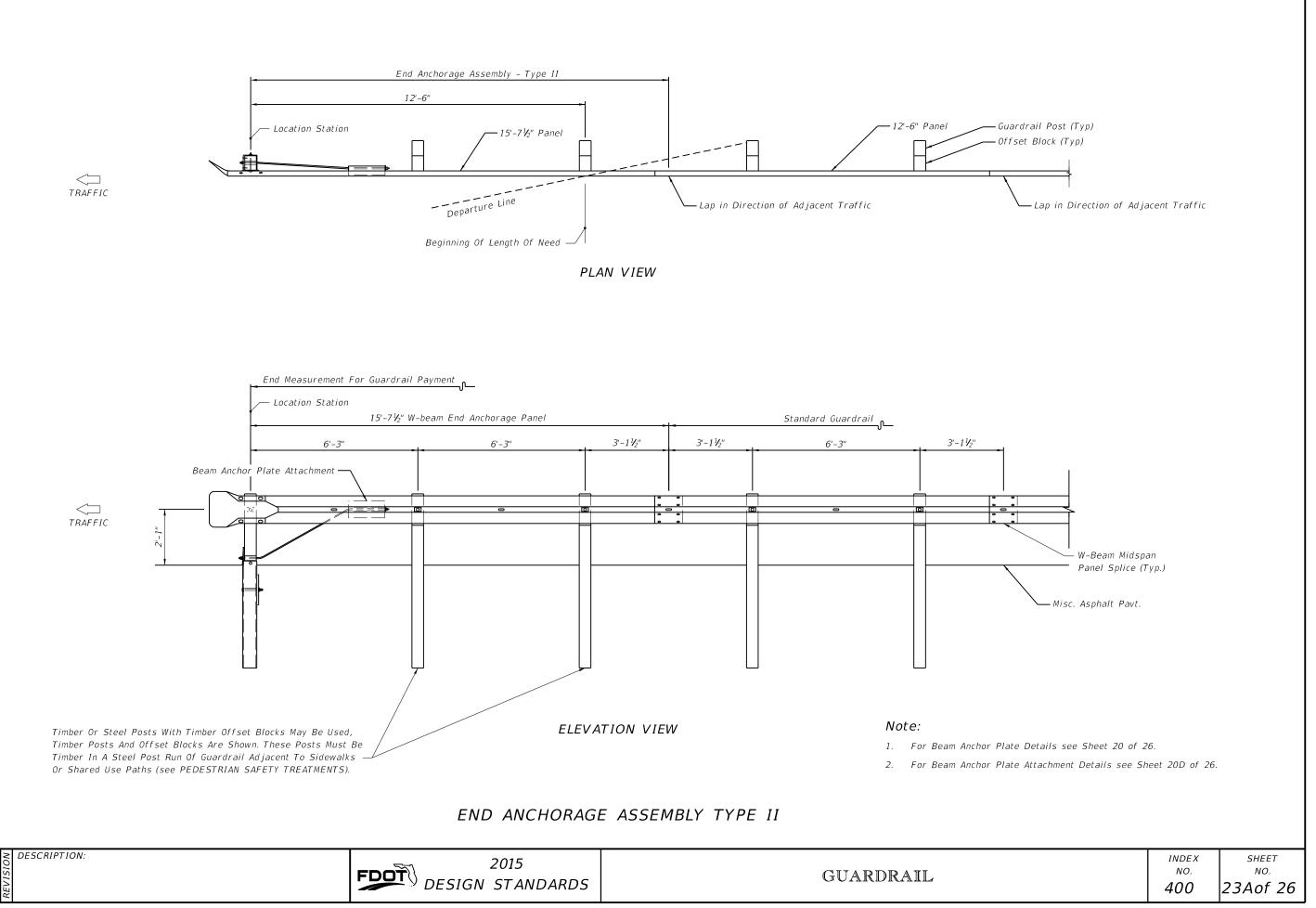
- grouted with neat finish.
- cleaned when setting bolts and anchors and dry when setting wedge anchors.
- metalized in accordance with Section 562 of the Standard Specifications.









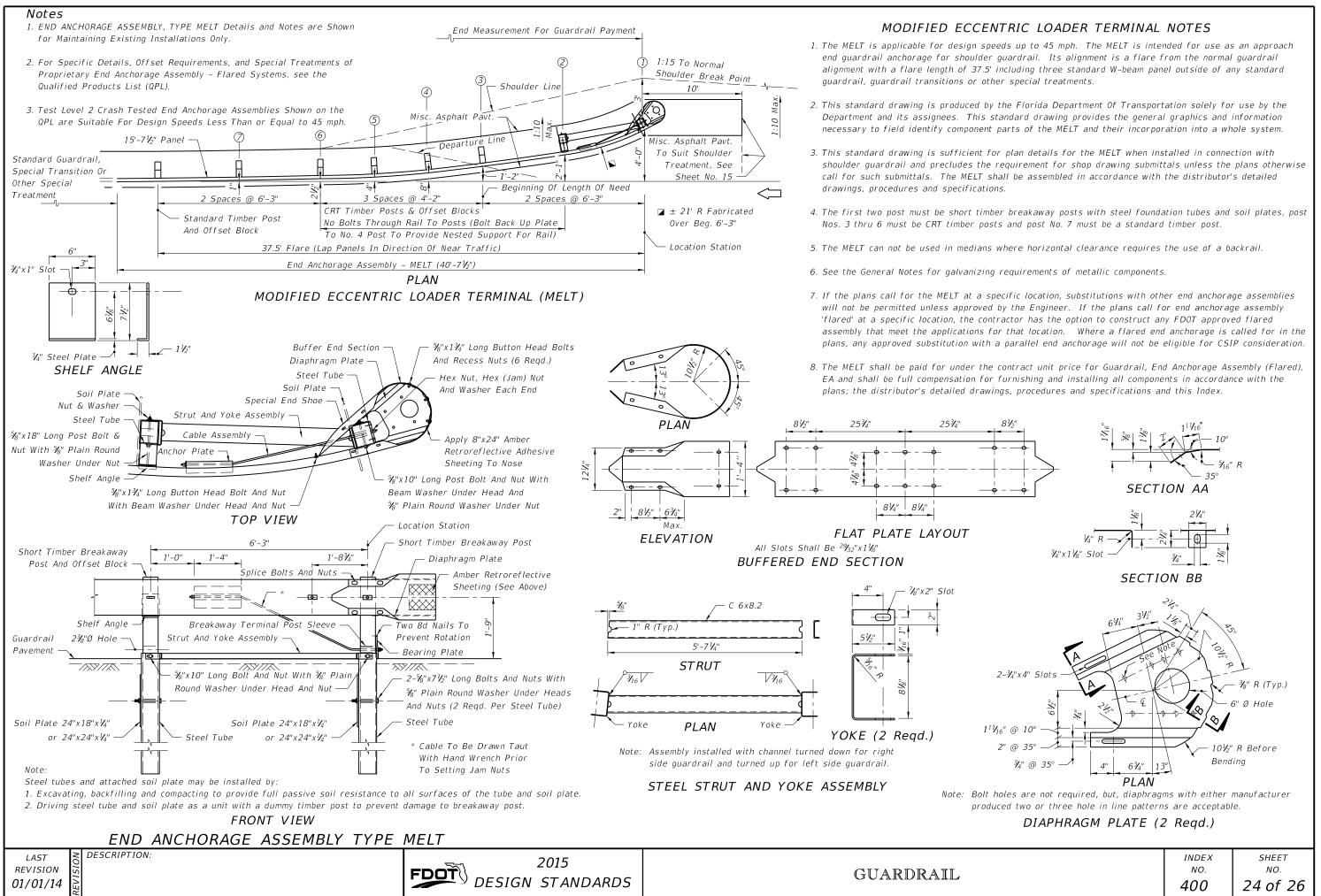


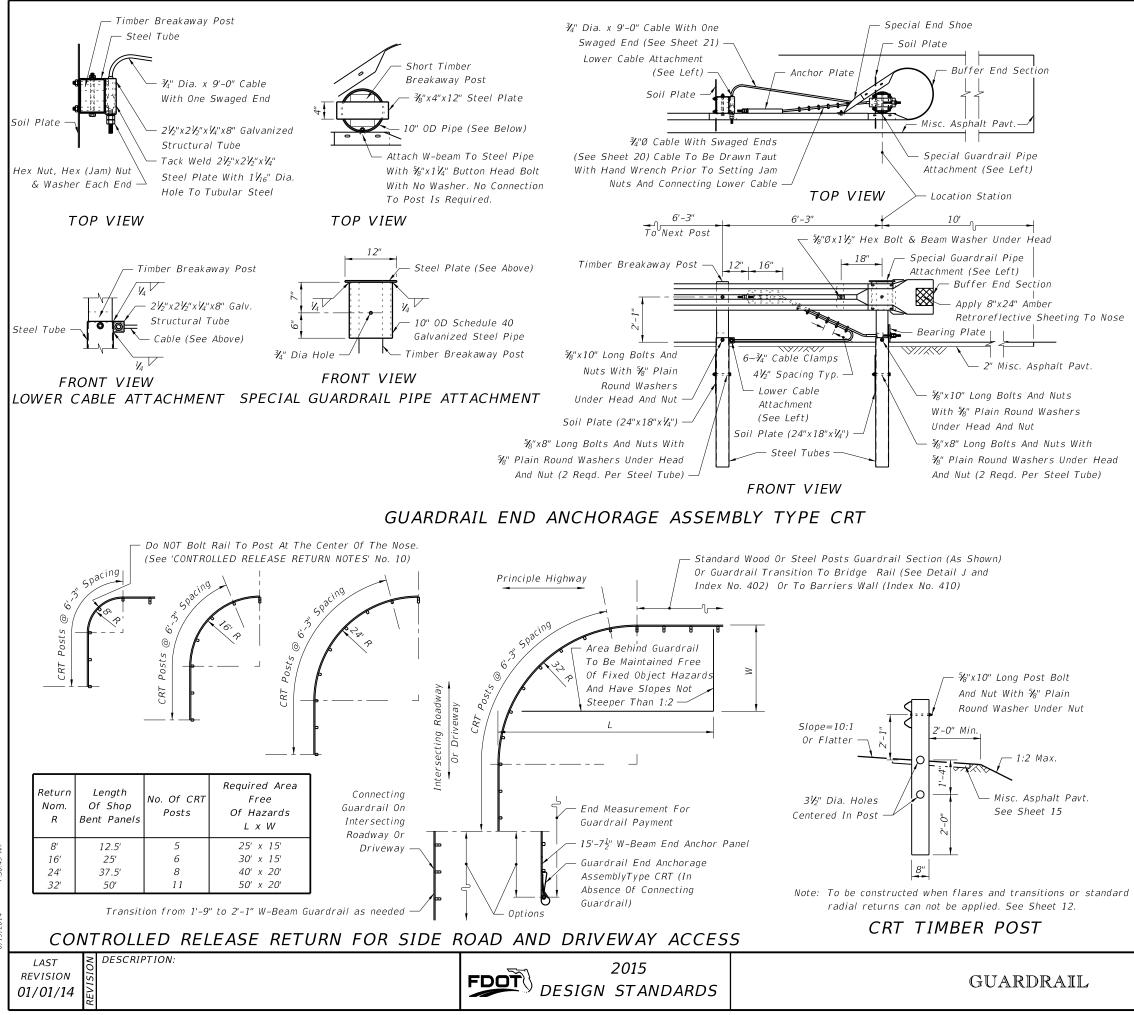
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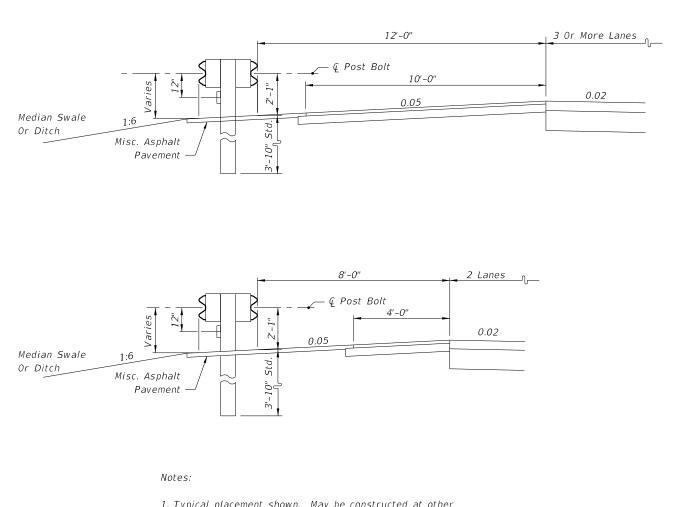


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# CONTROLLED RELEASE RETURN NOTES

- Controlled release returns are intended for use (a) in openings in continuous guardrail for driveway and side road access when flares and transitions or standard radial returns can not be applied (Sheet 12); and, (b) for shielding the ends of bridge traffic rails and barrier walls where the driveway and side road access is in close proximity to the structure and space does not permit the proper use of approved flared and parallel types of Guardrail End Anchorage Assemblies.
- 2. Controlled release returns are not intended as a substitute or replacement for the appropriate use of approved vehicle impact attenuators.
- 3. Controlled release returns with either 8', 16' or 24' radii are designed for highway speeds of 60 mph or less.
- 4. The controlled release returns shown are designed as full returns based on an intersection angle of 90°. The return can be terminated with the Guardrail End Anchorage Assembly Type CRT or connected to standard guardrail as shown or as otherwise detailed in the plans.
- 5. The Guardrail End Anchorage Assembly Type CRT is to be used only for the controlled release returns with 8', 16', 24' and 32' radii as shown; the assembly is not to be used in any tangent rail or flared rail applications. Other types of end anchorage assemblies are not to be used in the controlled release returns.
- 6. The area immediately behind the control release return shall have slopes not steeper than 1:2 and be maintained free of fixed objects in accordance with the area limits tabulated in the plan below.
- 7. The surface approaching the controlled release return shall have a transverse slope not exceeding 1:10. The effective width of the transverse surface is to be based on standard vehicle departure, return radii and preceding shielding; the width (beyond shoulder) shall be not greater than the corresponding 15' and 20', 'W' values tabulated below.
- 8. The curved guardrail portion of the controlled release return shall be full section shop bent panels (12.5' or 25' panels).
- 9. Washers are not to be used between the guardrail beam and the head of the button head post bolts at any controlled release terminal (CRT) post or at any Guardrail End Anchorage Assembly Type CRT breakaway timber post.
- 10. The guardrail beam of the 8' radius return is not bolted to the center control release post.
- 11. See the General Notes for galvanizing requirements of metallic components.
- 12. Controlled release return systems shall be paid for under the contract unit prices for Guardrail (Roadway), LF, Guardrail (Shop-bent Panels), LF, and Guardrail, End Anchorage Assembly (Type CRT), EA as called for in the plans or by permit and shall be full compensation for furnishing and installing all components in accordance with the plans and with this index. CRT posts are included in the cost for guardrail.

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NO.	NO.
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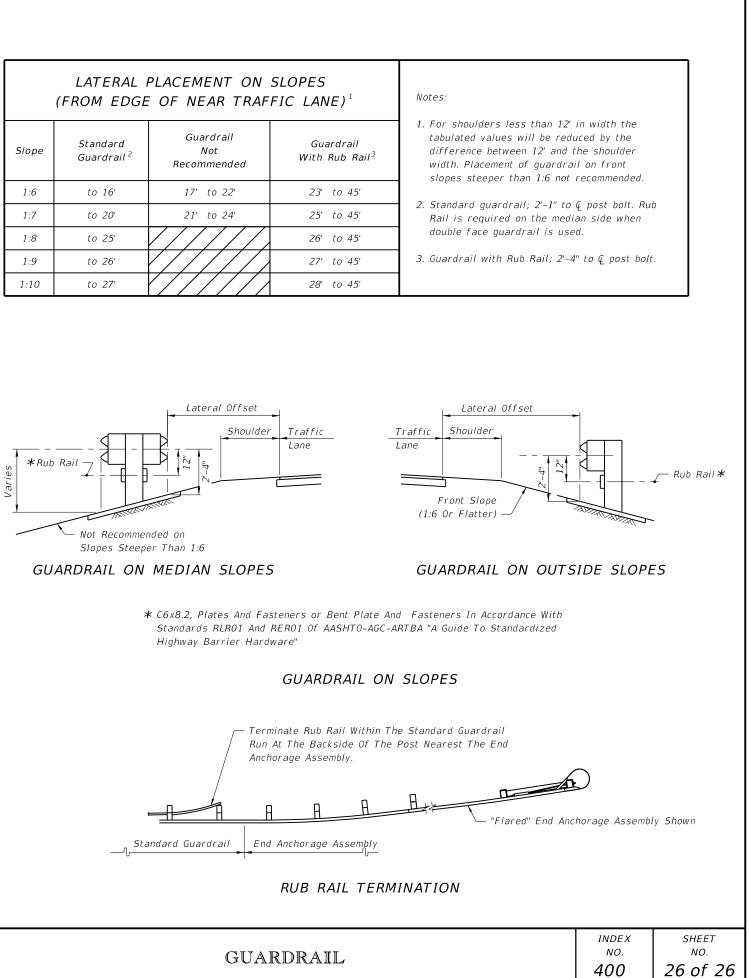


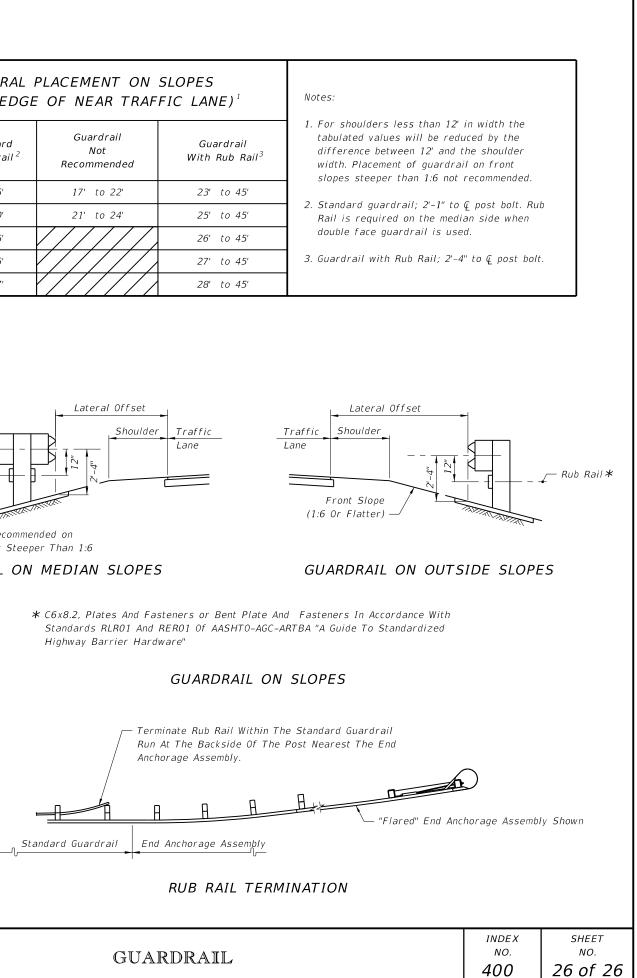
- 1. Typical placement shown. May be constructed at other locations as called for in the plans.
- 2. Rub Rail is required on the median side or ditch side of the barrier.

### MOUNTING HEIGHT FOR DOUBLE FACED GUARDRAIL ON MEDIAN SHOULDERS (FREEWAYS)

# LATERAL PLACEMENT ON SLOPES

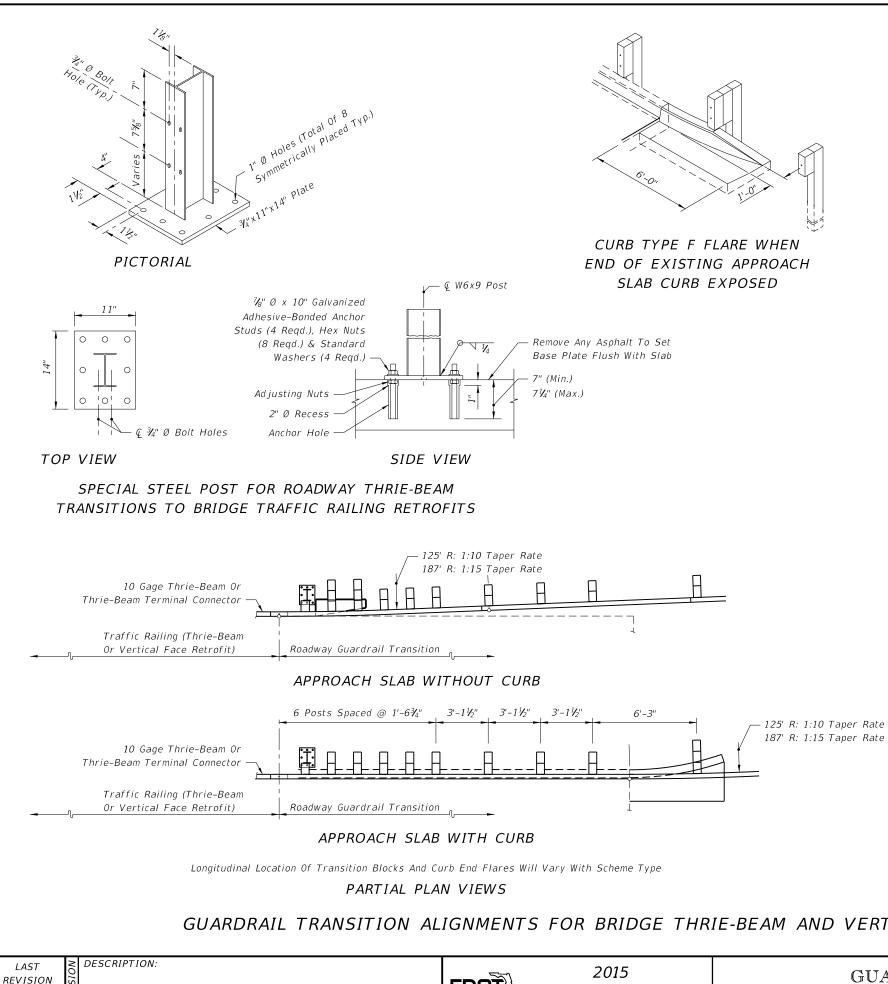
Slope	Standard Guardrail <sup>2</sup>	Guardrail Not Recommended	Guardrail With Rub Rail <sup>3</sup>
1:6	to 16'	17' to 22'	23' to 45'
1:7	to 20'	21' to 24'	25' to 45'
1:8	to 25'		26' to 45'
1:9	to 26'		27' to 45'
1:10	to 27'	(/////	28' to 45'





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- with safety shaped traffic railing.
- (IDS-470 & IDS-480).
- provided on this index, refer to Index No. 400.

# NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- curb blunt ends are not in place.

# DESIGN NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT

LAST REVISION 07/01/13	GUARDRAIL TRANSITIONS ANDINDEX NO.SHEE NO.CONNECTIONS FOR EXISTING BRIDGES4021 of	
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### GENERAL NOTES

1. This index provides thrie-beam transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 23 apply to bridges with retrofitted traffic railings, (Sheet 23 shows the trailing end guardrail connections). Sheet 24 applies to bridges

2. The schemes identified by Arabic numerals in this index are complementary to the bridge traffic railing barrier retrofit schemes with like numeral identification in Index Nos. 470, 471 through 476, 480 through 483. The schemes in this index identified by Roman numerals are complementary to bridge safety shaped traffic railing barrier where determined to be in accordance with applications of criteria specified in the Instructions for Design Standards

3. For guardrail applications and details of related hardware and accessories that are not

1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.

2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to

3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated using ASTM A36 or ASTM A709 Grade 36 steel. Welding shall conform to ANSI/AASHTO/AWS D1.5. The assembly shall be hot-dip zinc coated in accordance with Section 536 of the Specifications.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

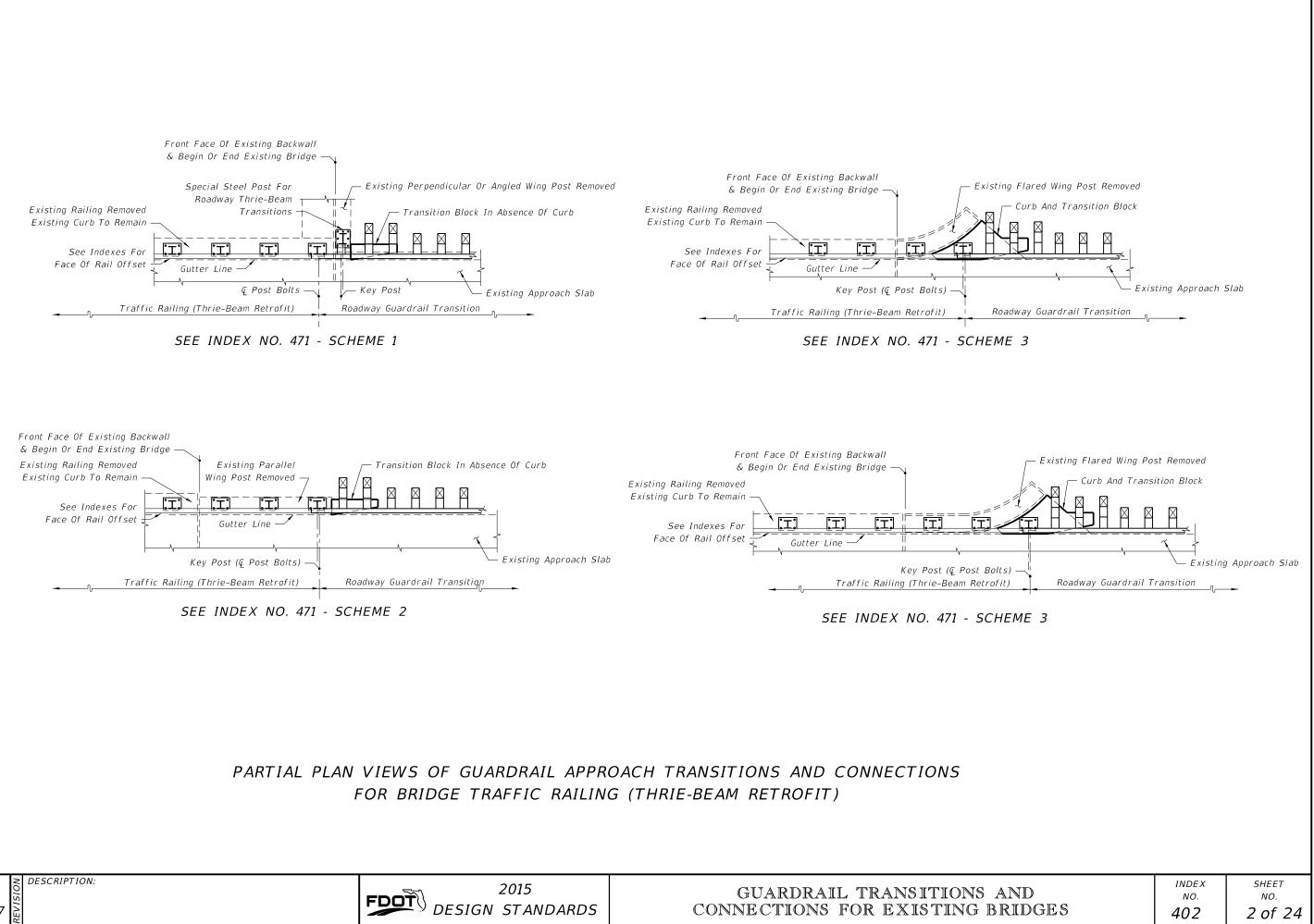
4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

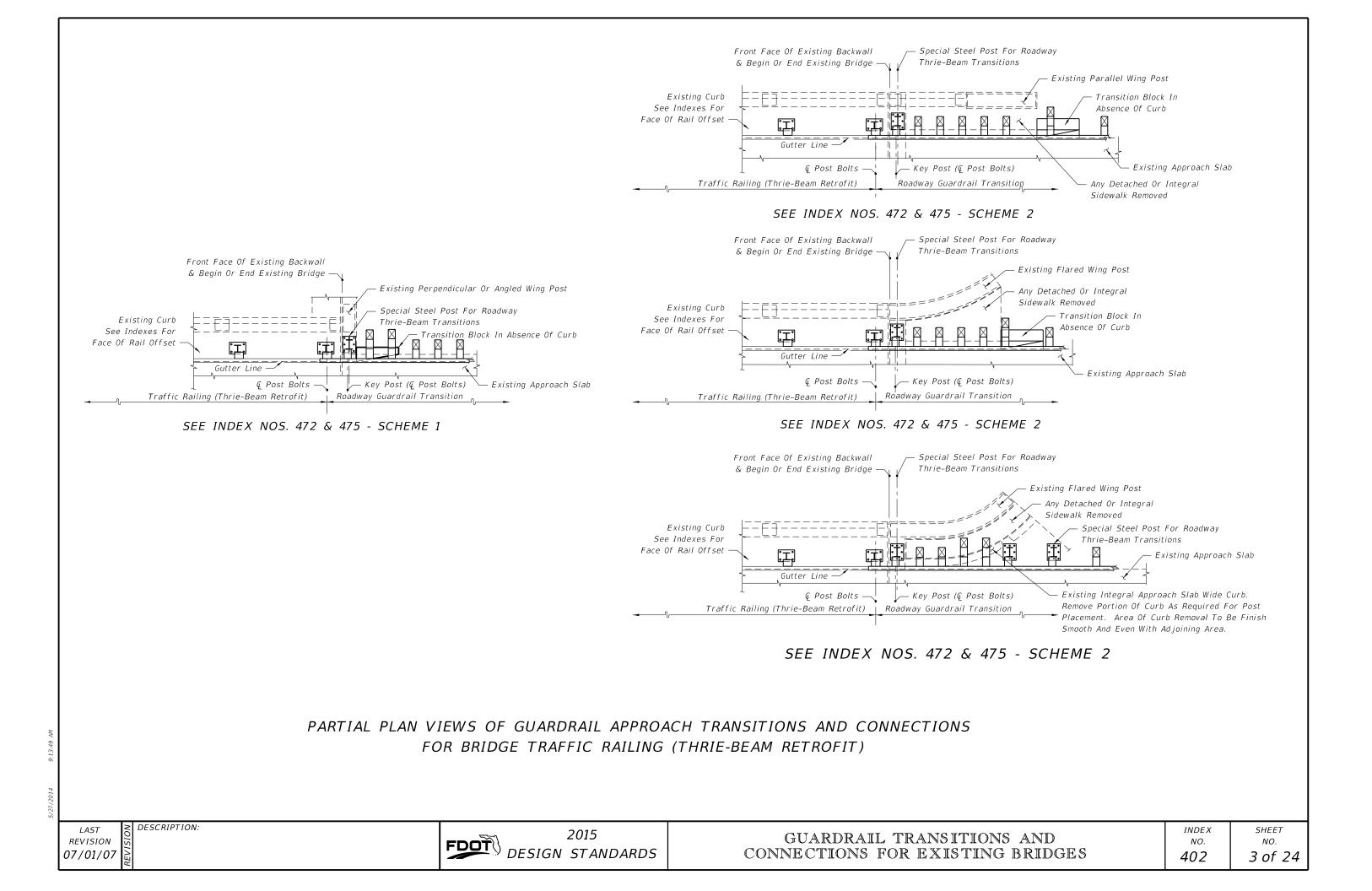
5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 12 through 15 and the flag notation on Sheet 23.

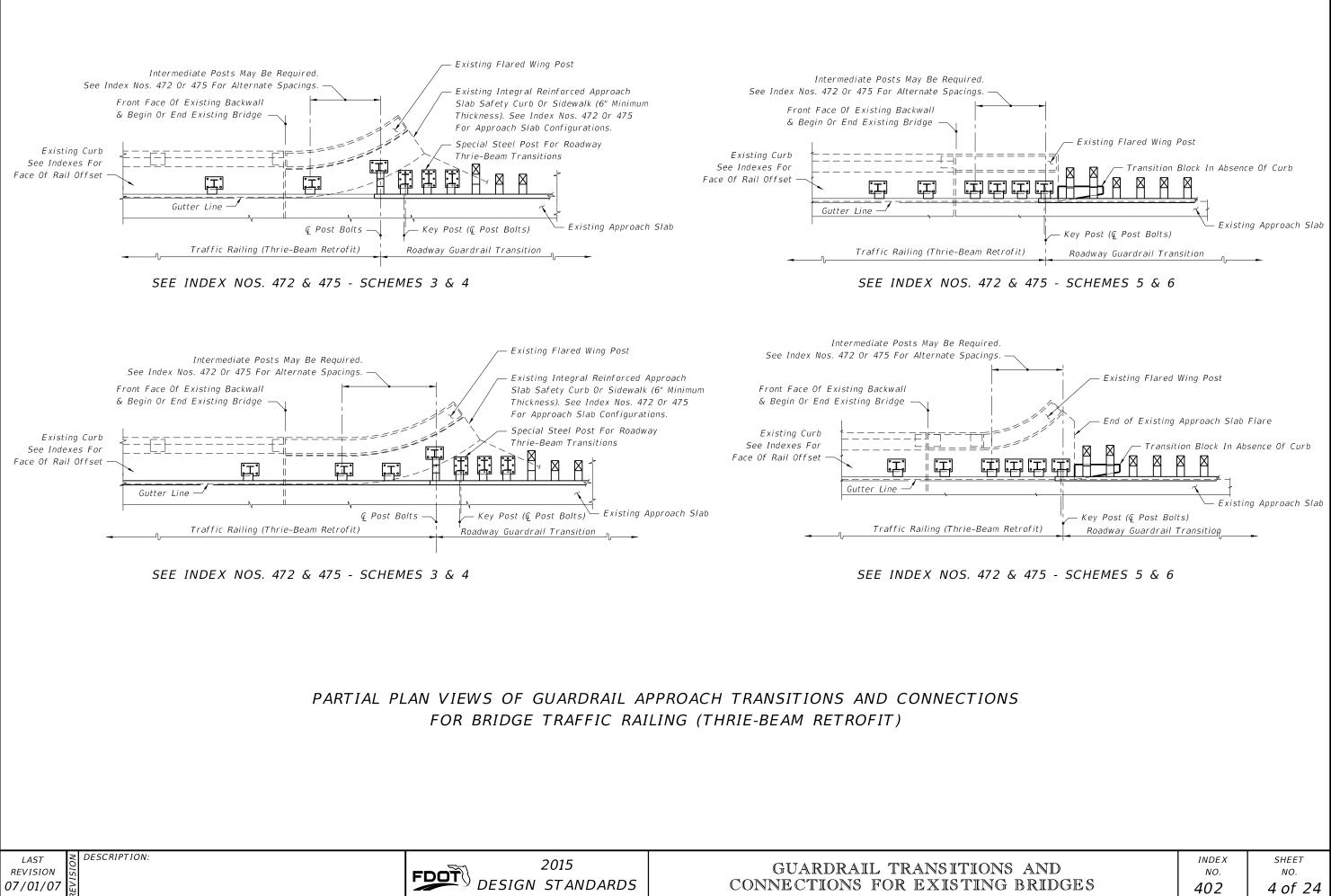
6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

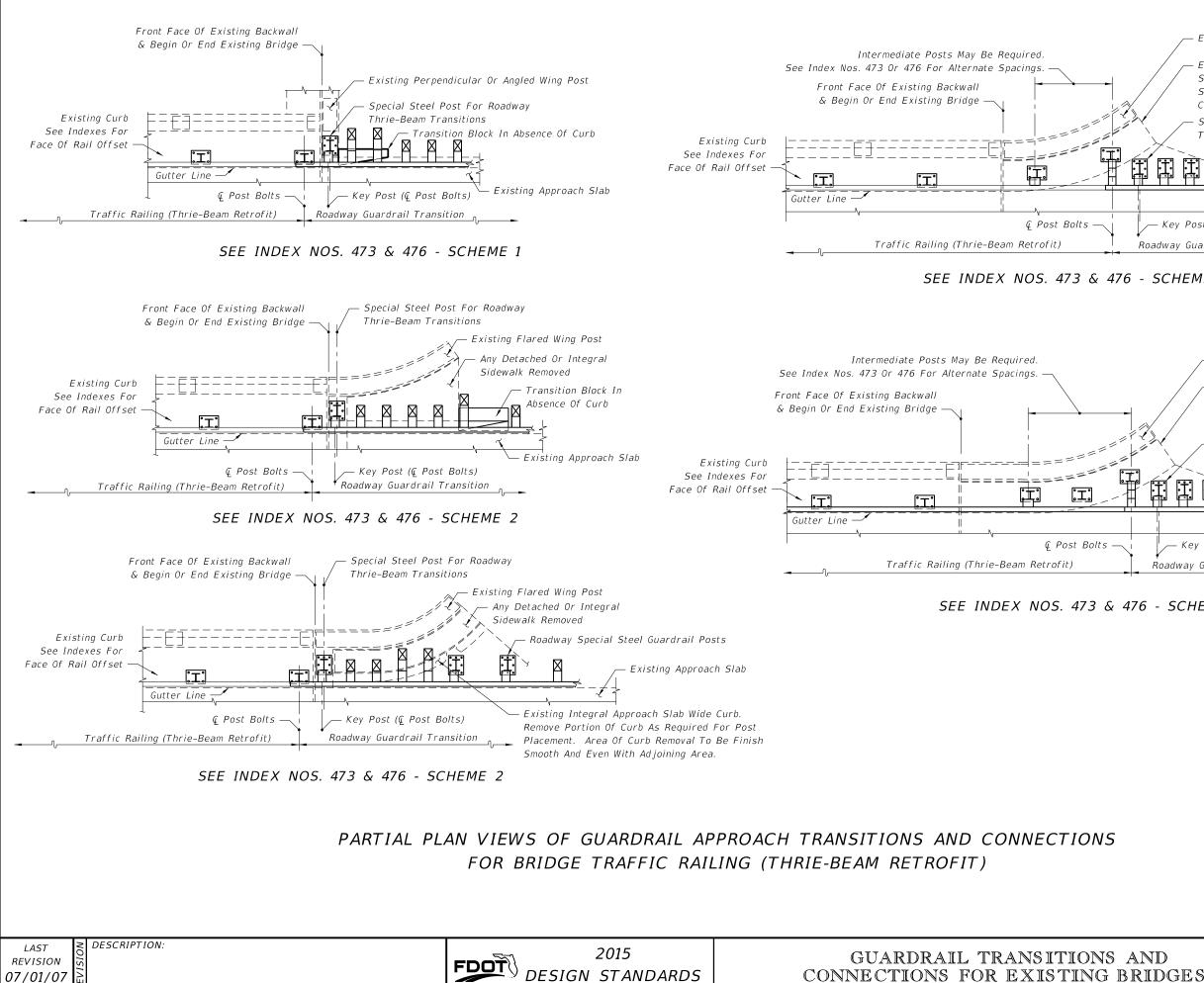
1. For selection of an appropriate transition scheme, see the Instructions for Design Standards (IDS-470 & IDS-480) for instructions to the Structures and Roadway engineers.



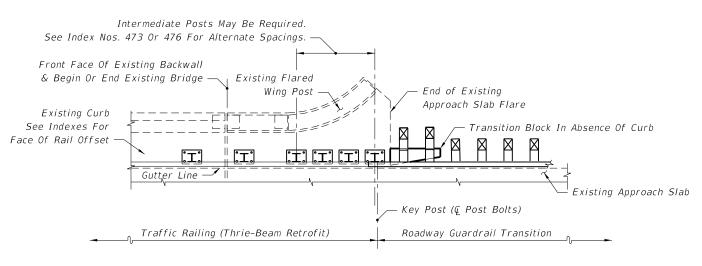
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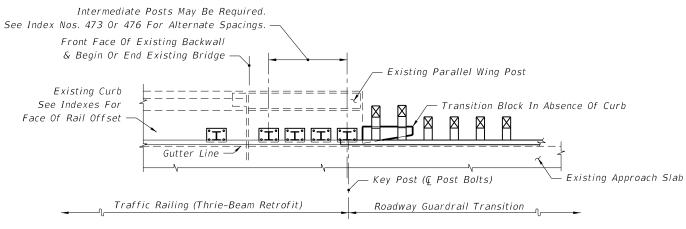




Existing Flared Win	g Post	
Existing Integral Re Safety Curb Or Side See Index Nos. 473 Configurations.	ewalk (6" Minim	um Thickness).
Special Steel Post Thrie-Beam Transit		
	2 I	
Key Post (& Post Bolts)	Existing A	pproach Slab
Roadway Guardrail Transition	<b>_</b>	
476 - SCHEMES 3 & 4	·	
— Existing Flared	Wing Post	
Existing Integra Safety Curb Or See Index Nos. Configurations.	Sidewalk (6" Mi	nimum Thickness).
Special Steel Po Thrie-Beam Tra		У
	×	
Key Post (& Post Bolts) Existing Approach Slab		
Roadway Guardrail Transitio	n	
x 476 - SCHEMES 3 & 4		
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SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

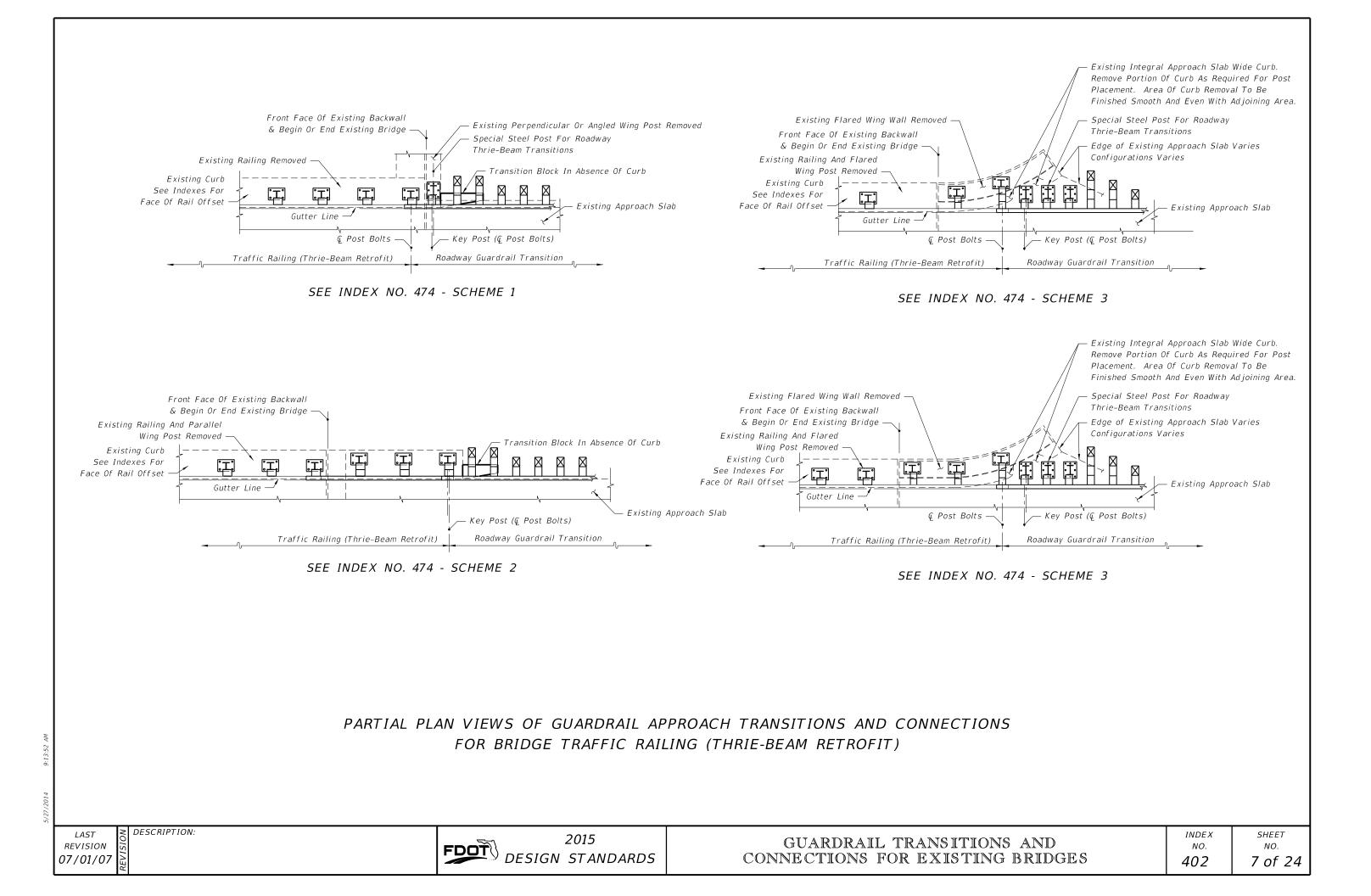


SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

## PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

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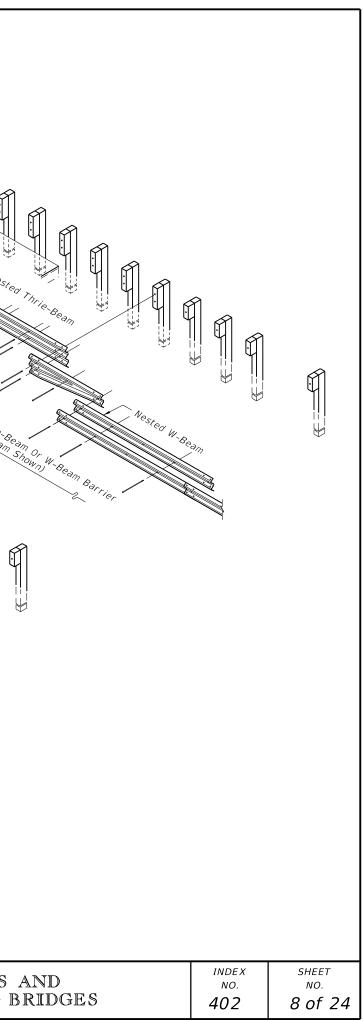
	INDEX	SHEET
S AND BRIDGES	NO. 402	6 of 24

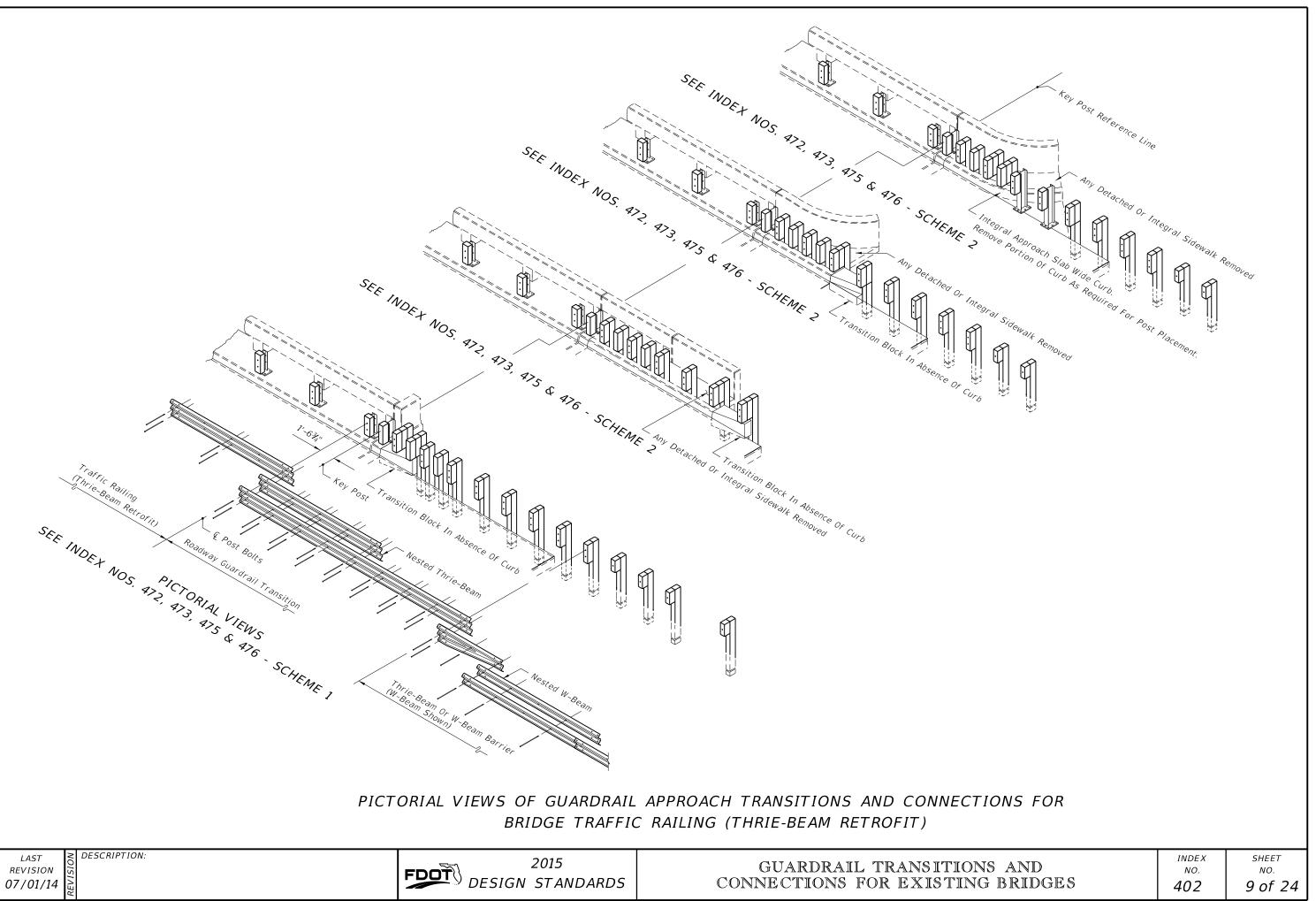


Trarie Reinouch Train Reinouch SEE INDEX NO. 471 VIEW SCHEME 1		
	ARDRAIL APPROACH TRANSIT TRAFFIC RAILING (THRIE-BEA	TIONS AND CONNECTIONS FOR AM RETROFIT)
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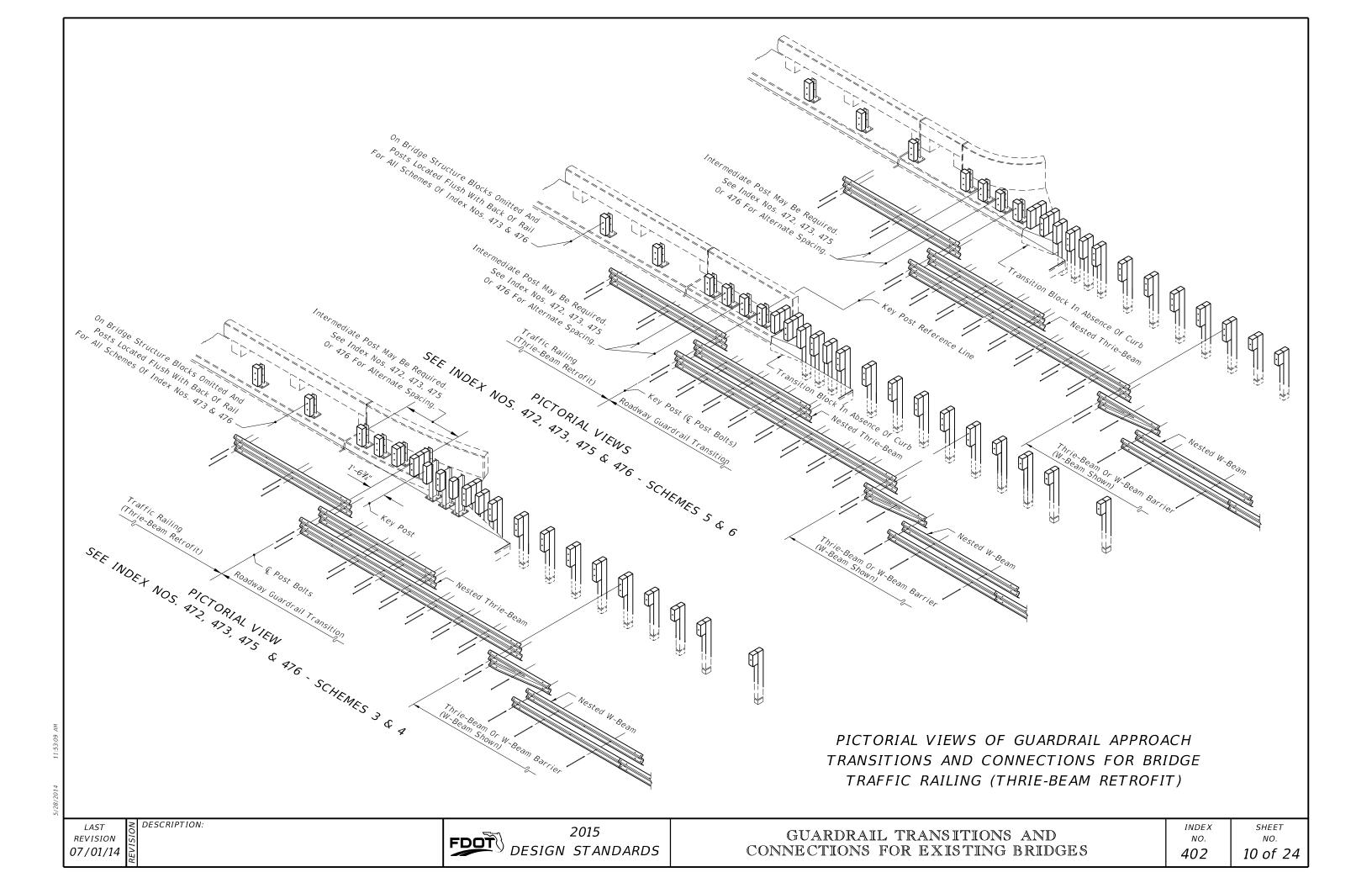
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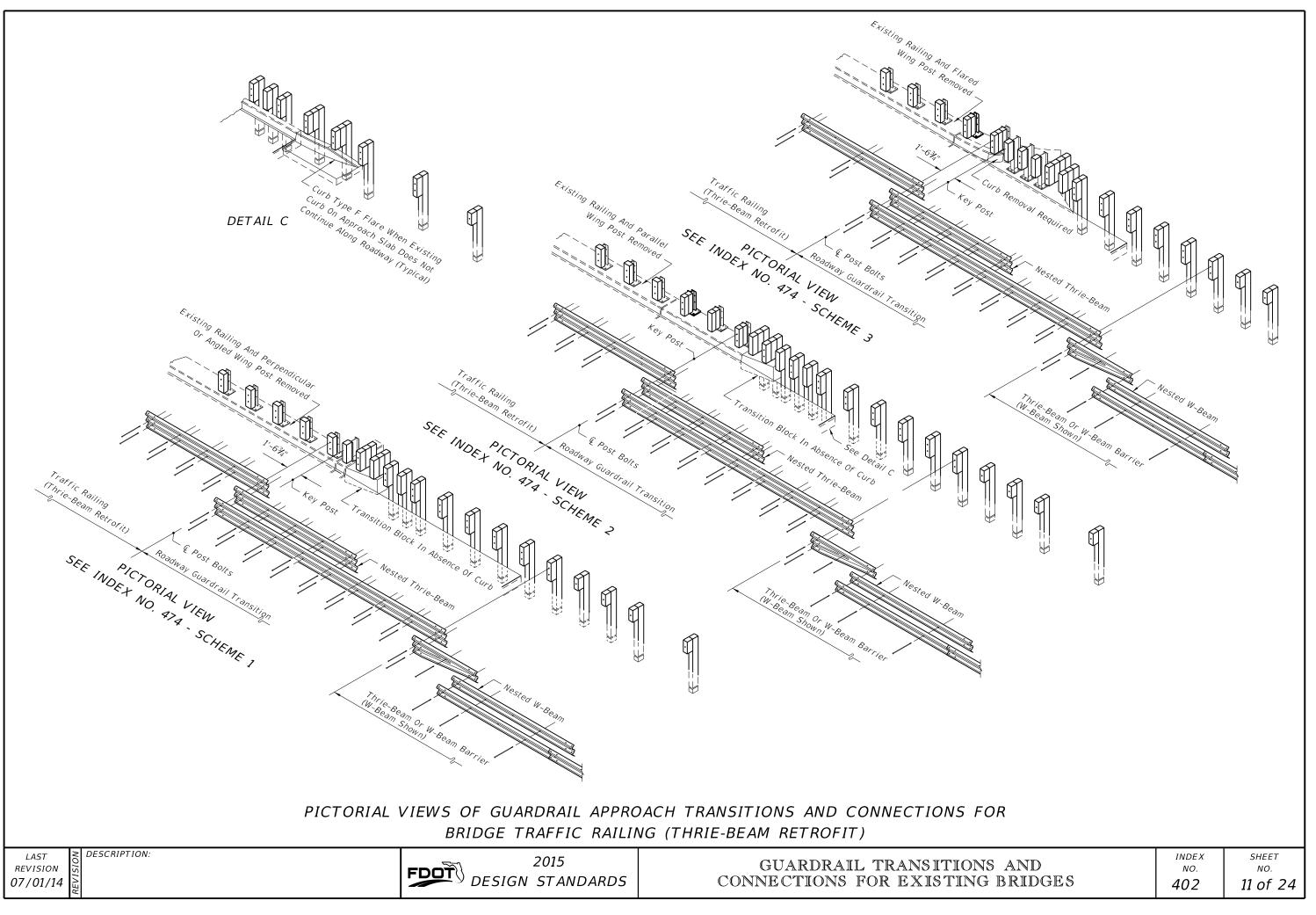
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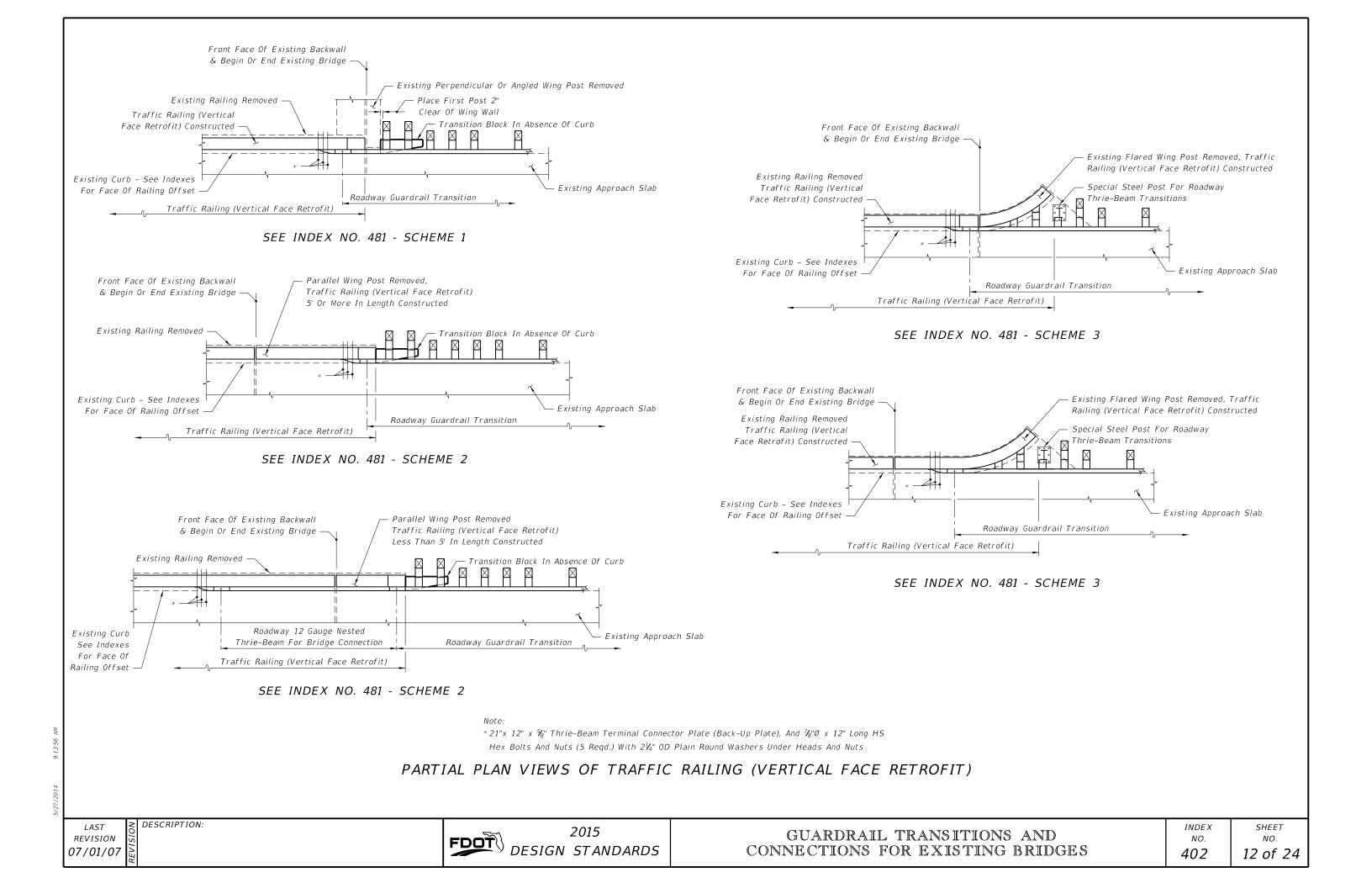


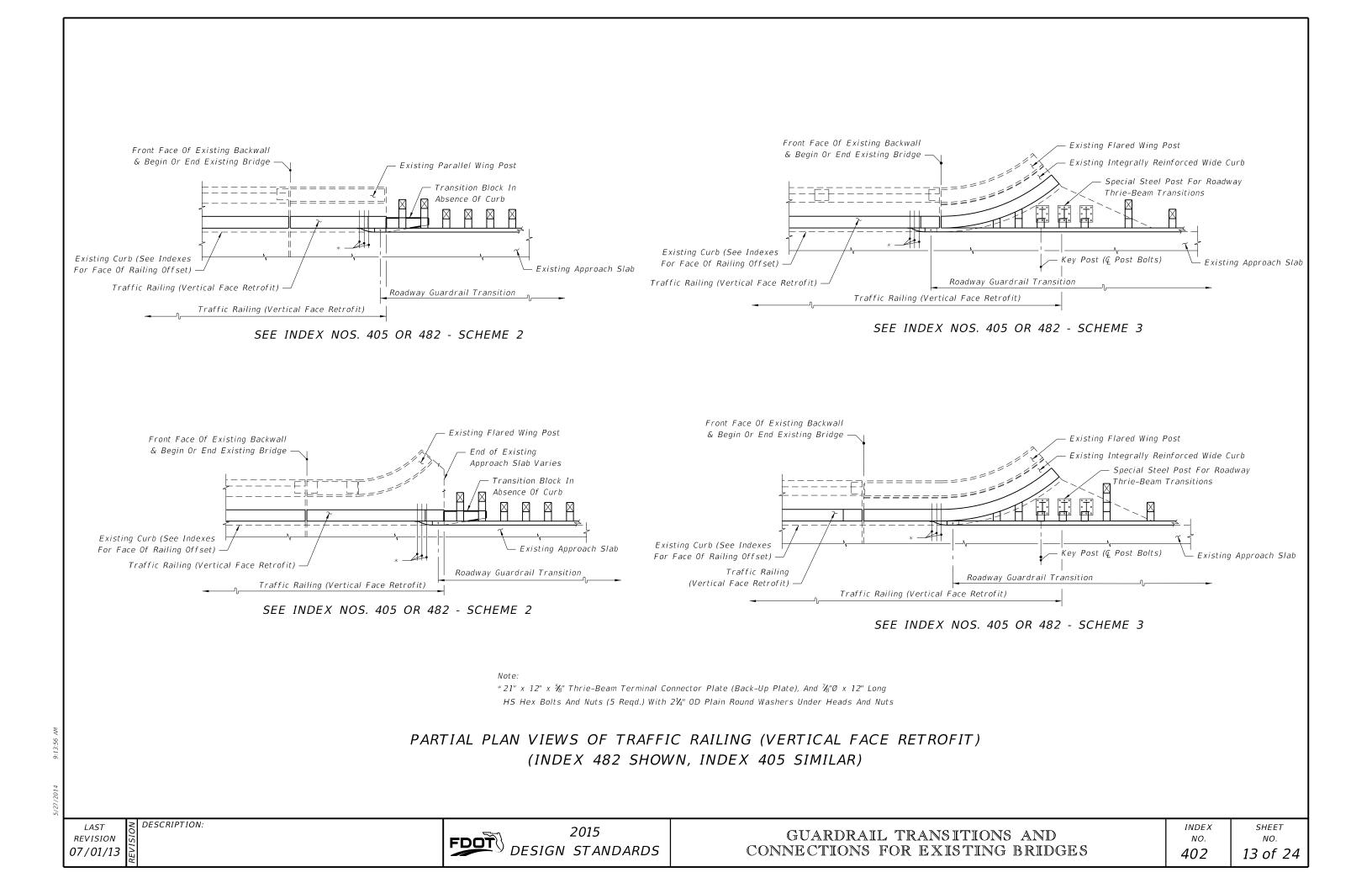
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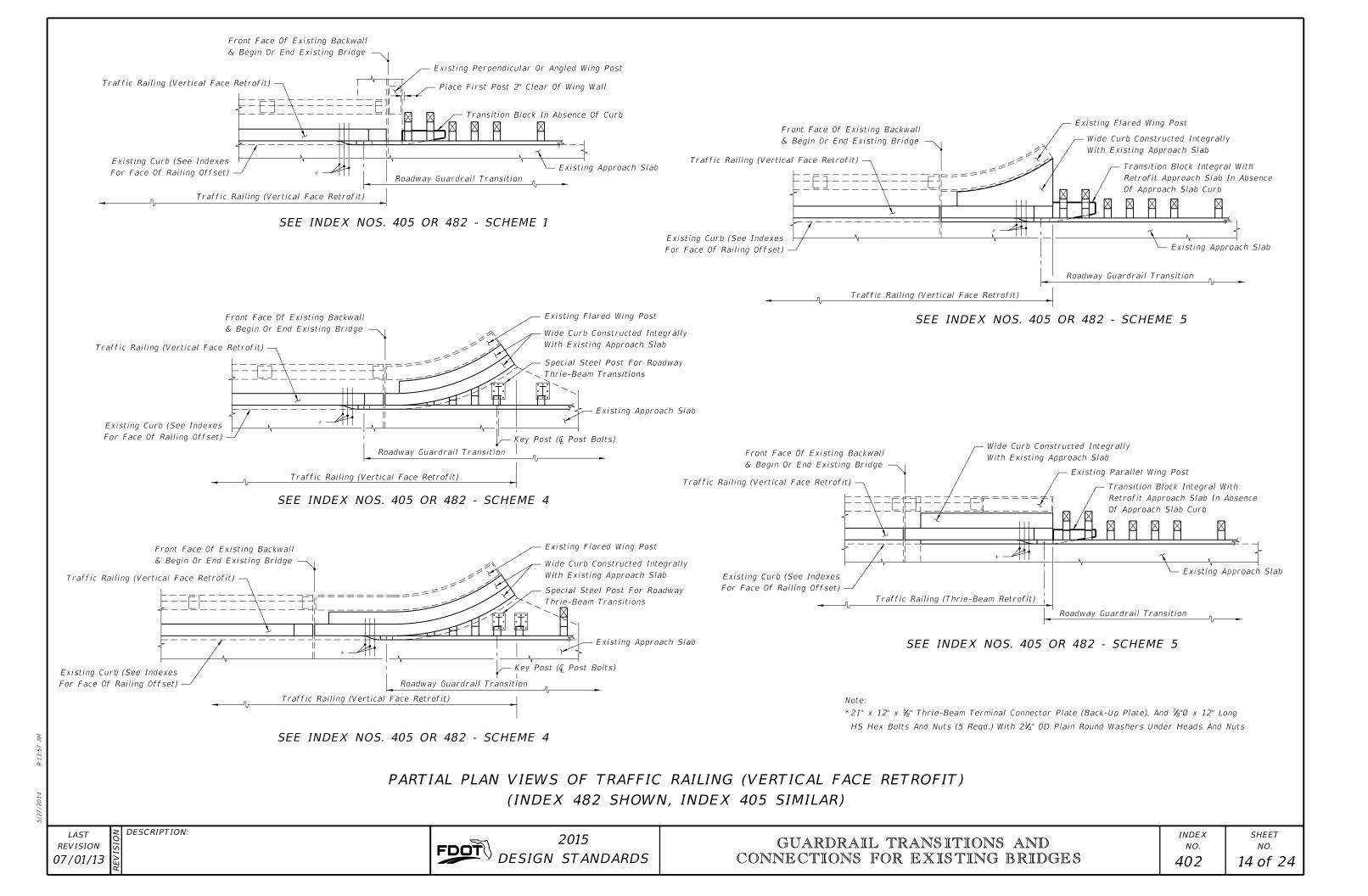




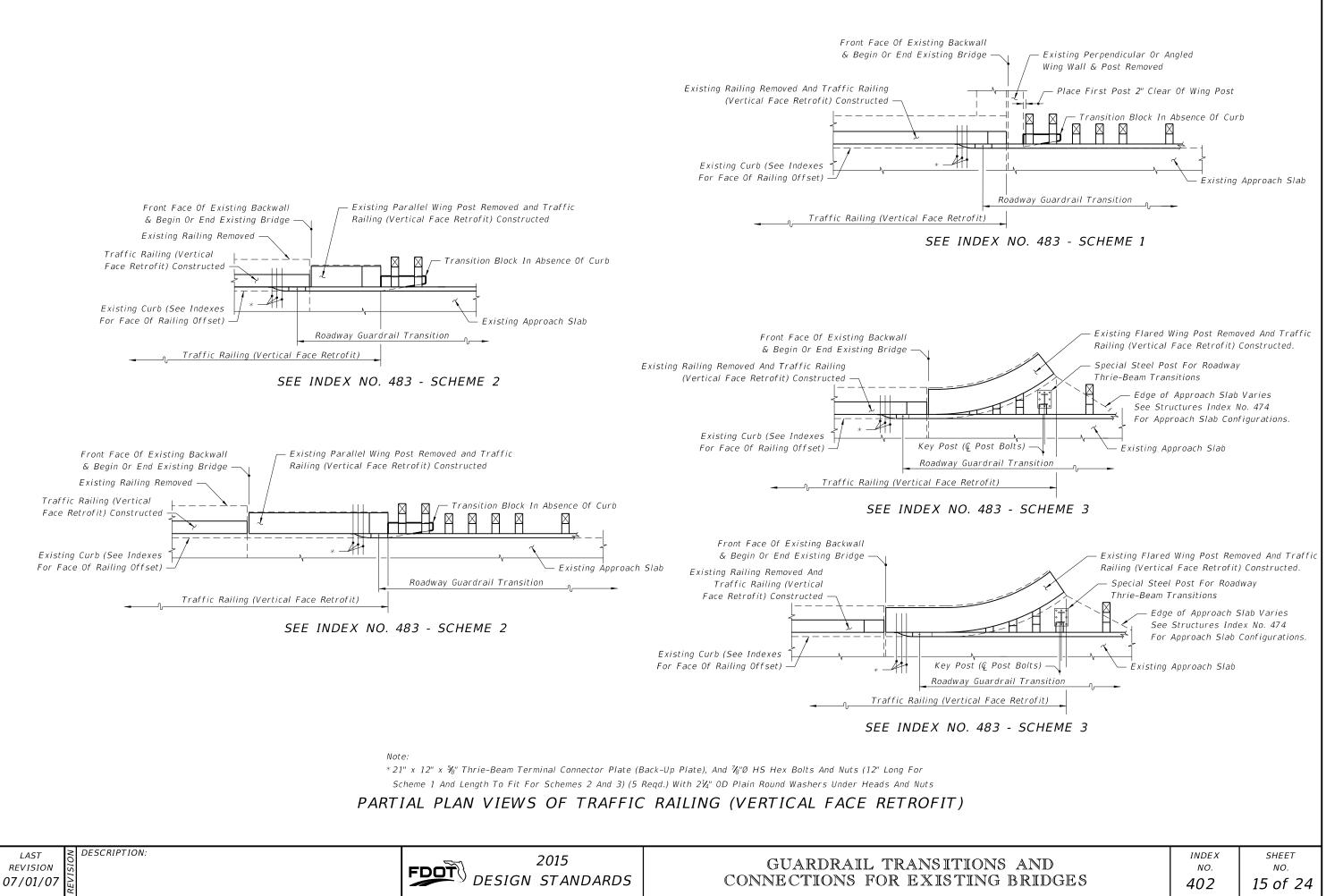
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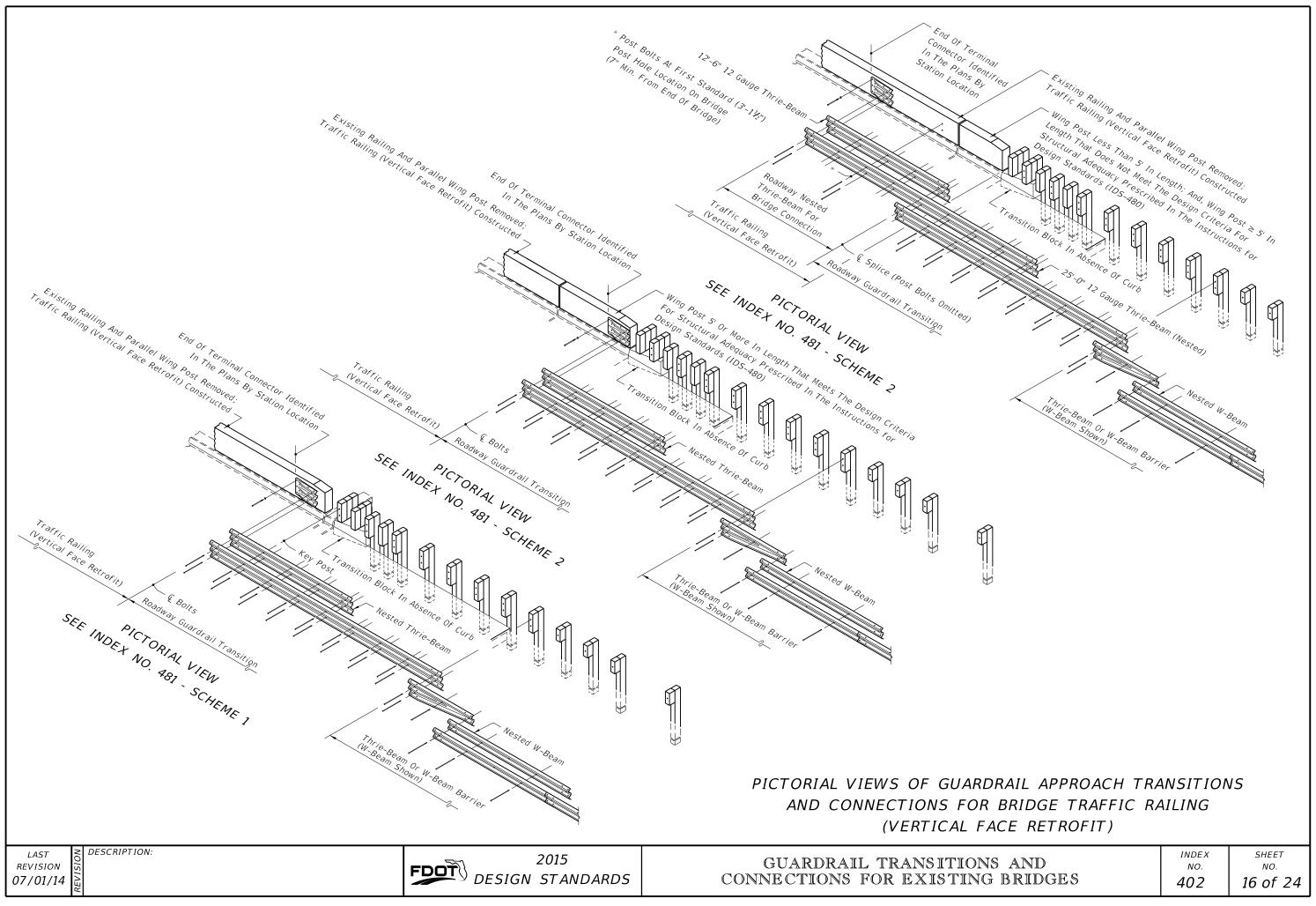


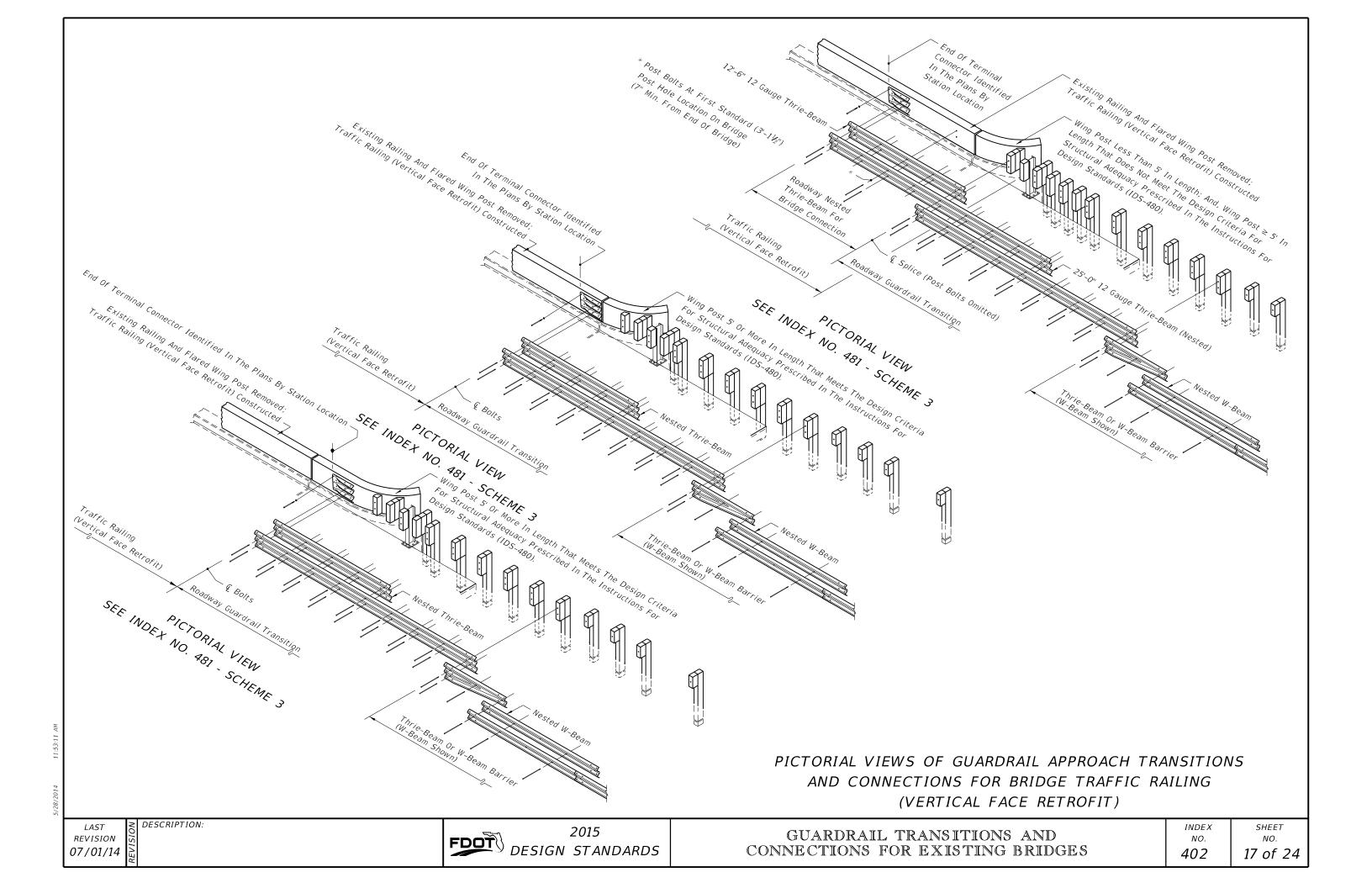


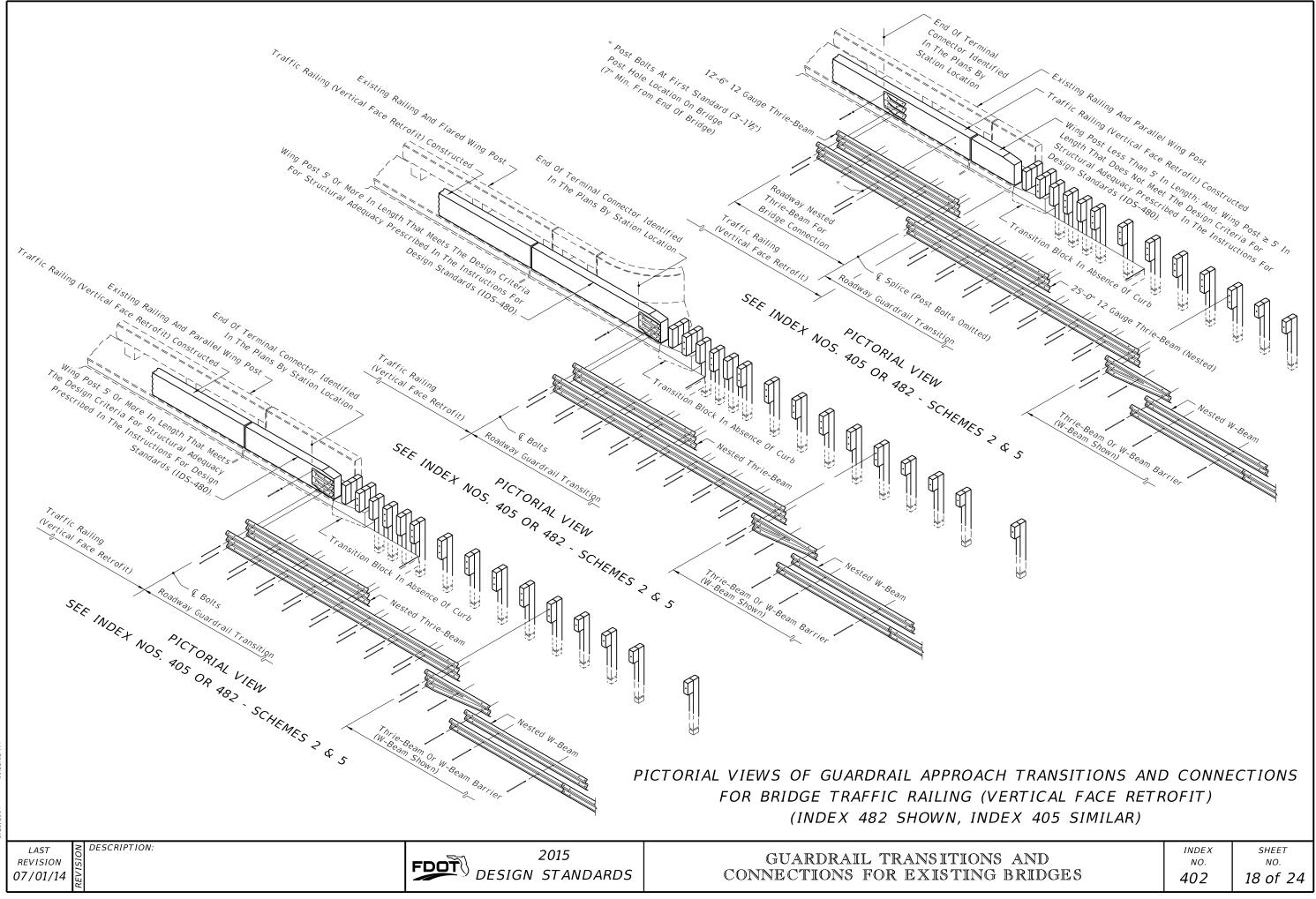


# & Begin Or End Existing Bridge

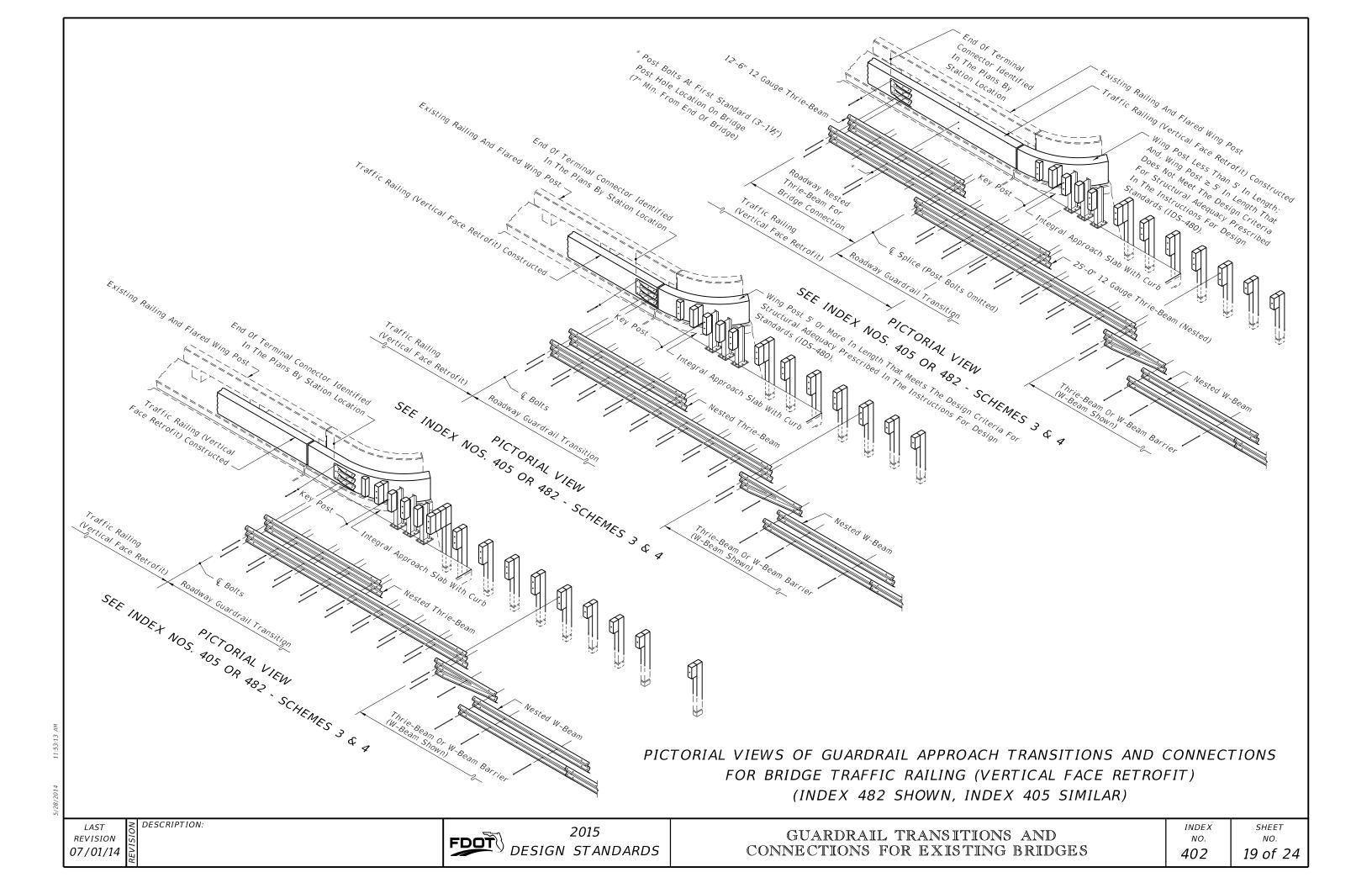


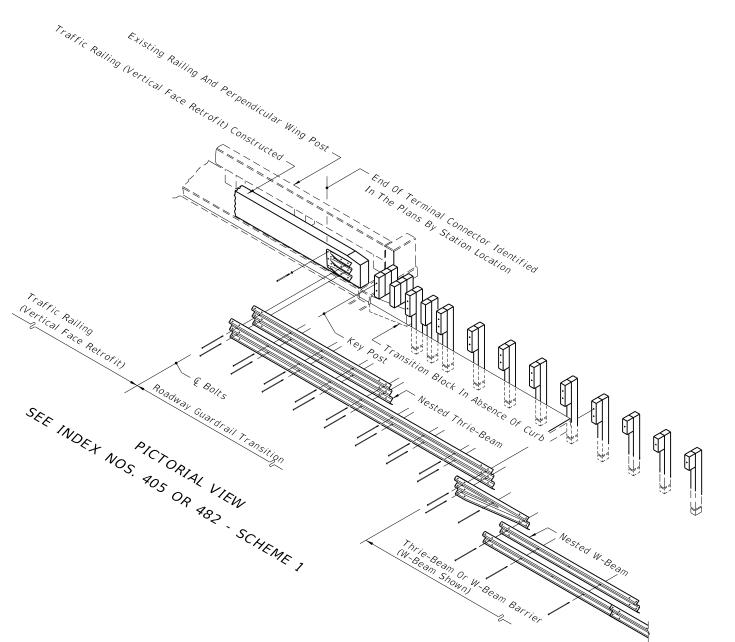






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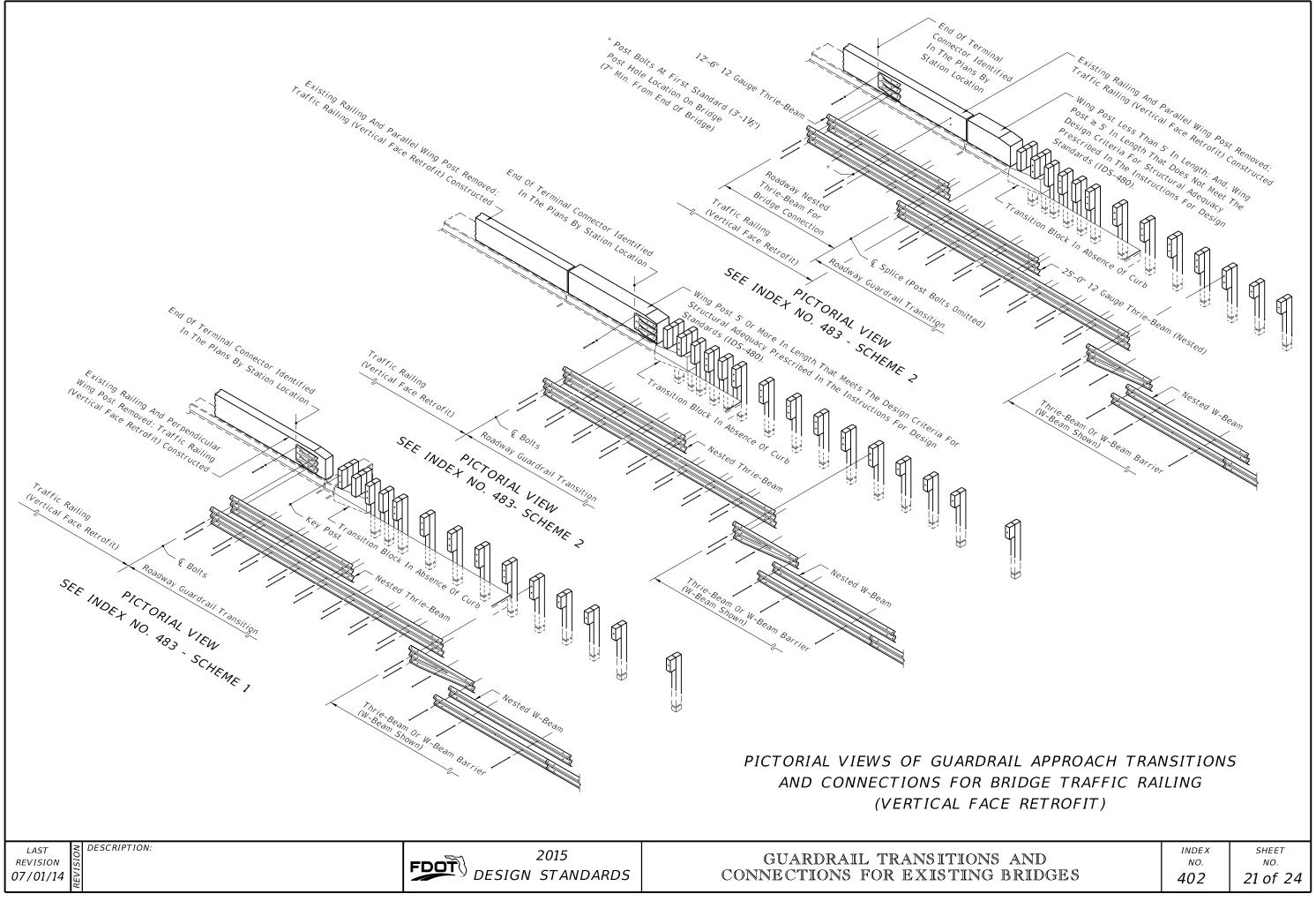


PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT) (INDEX 482 SHOWN, INDEX 405 SIMILAR)

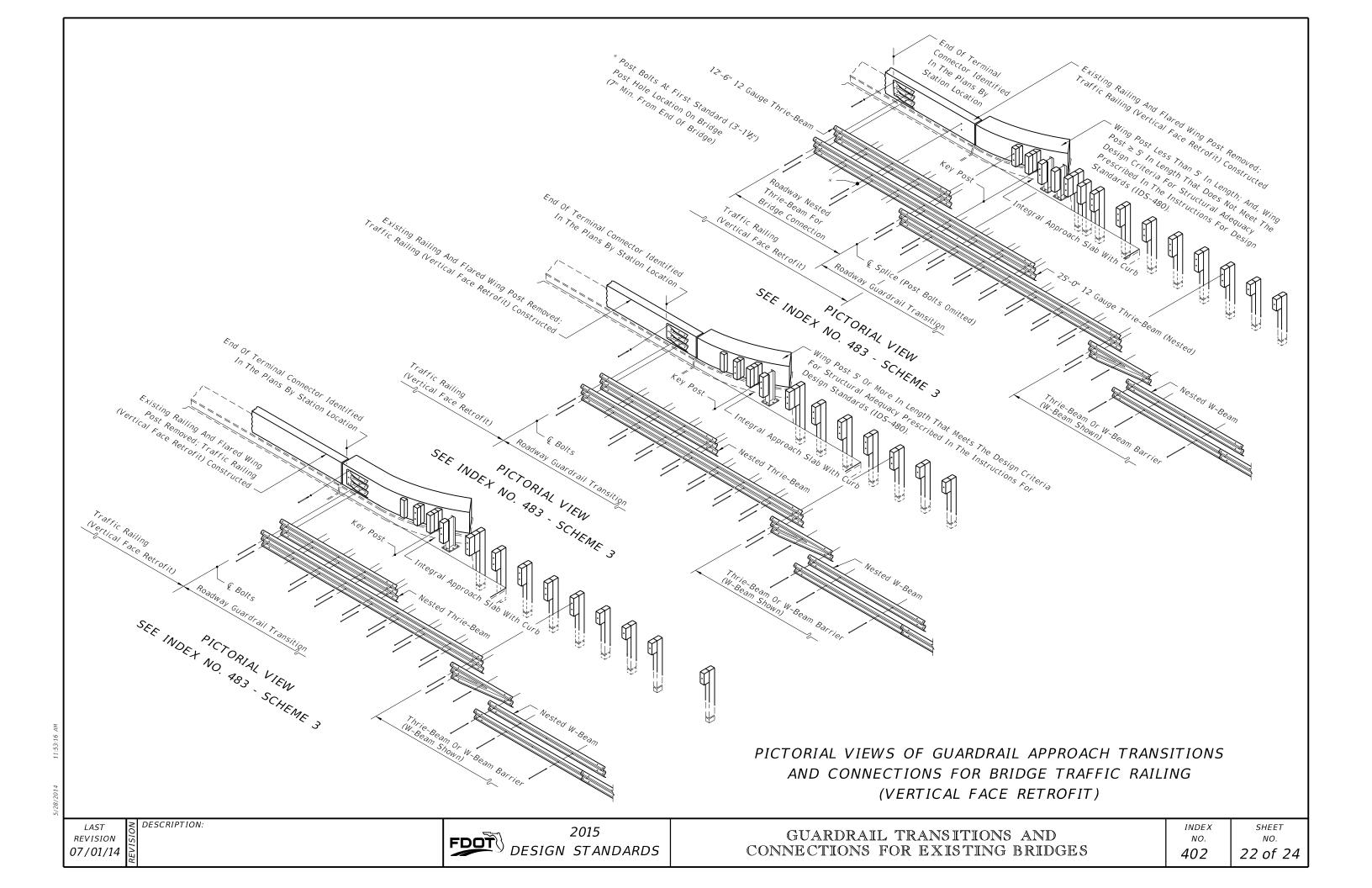
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GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES	index no. <b>402</b>	<sup>sнеет</sup> <sup>NO.</sup> <b>20 of 24</b>

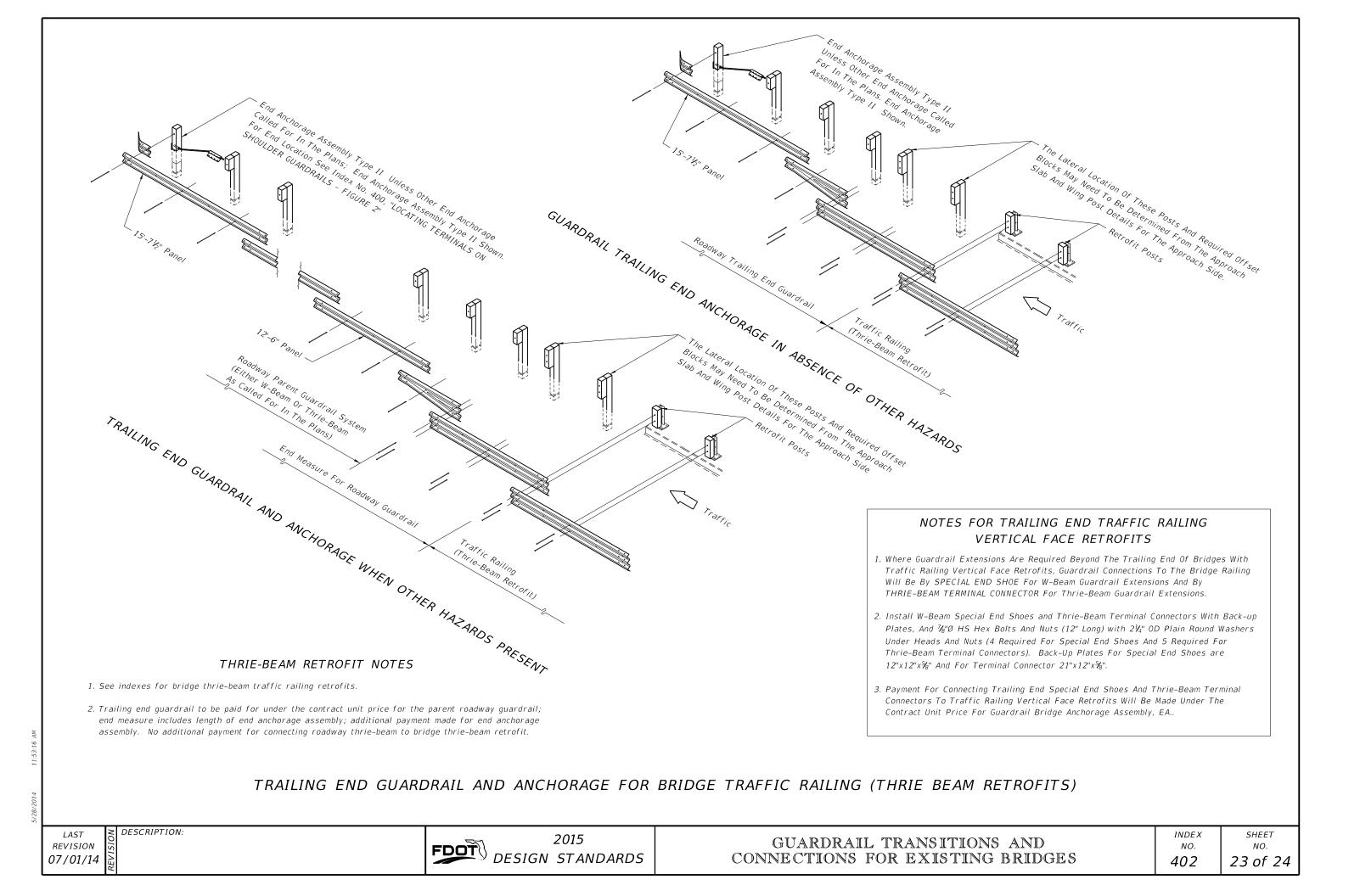
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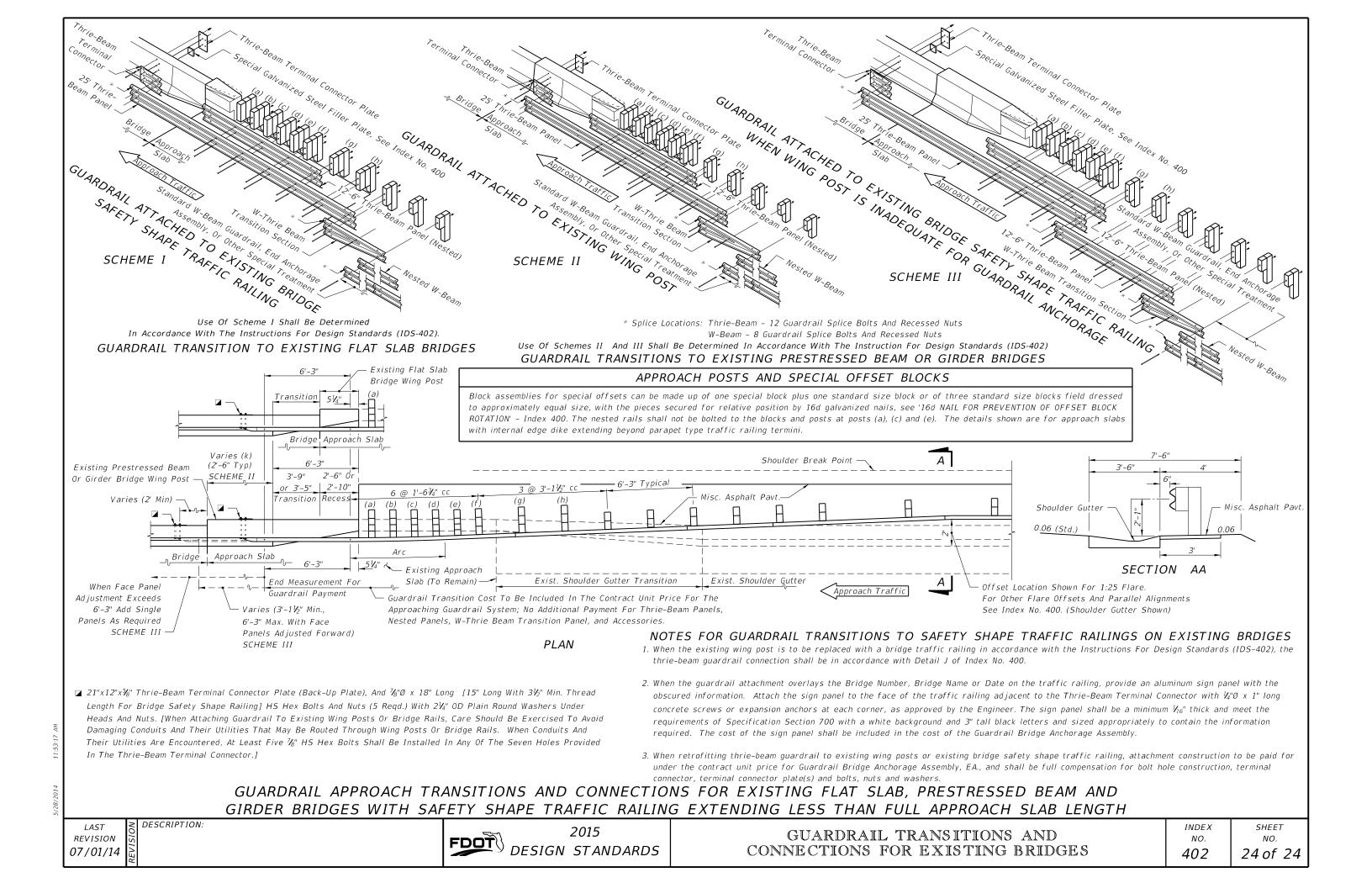
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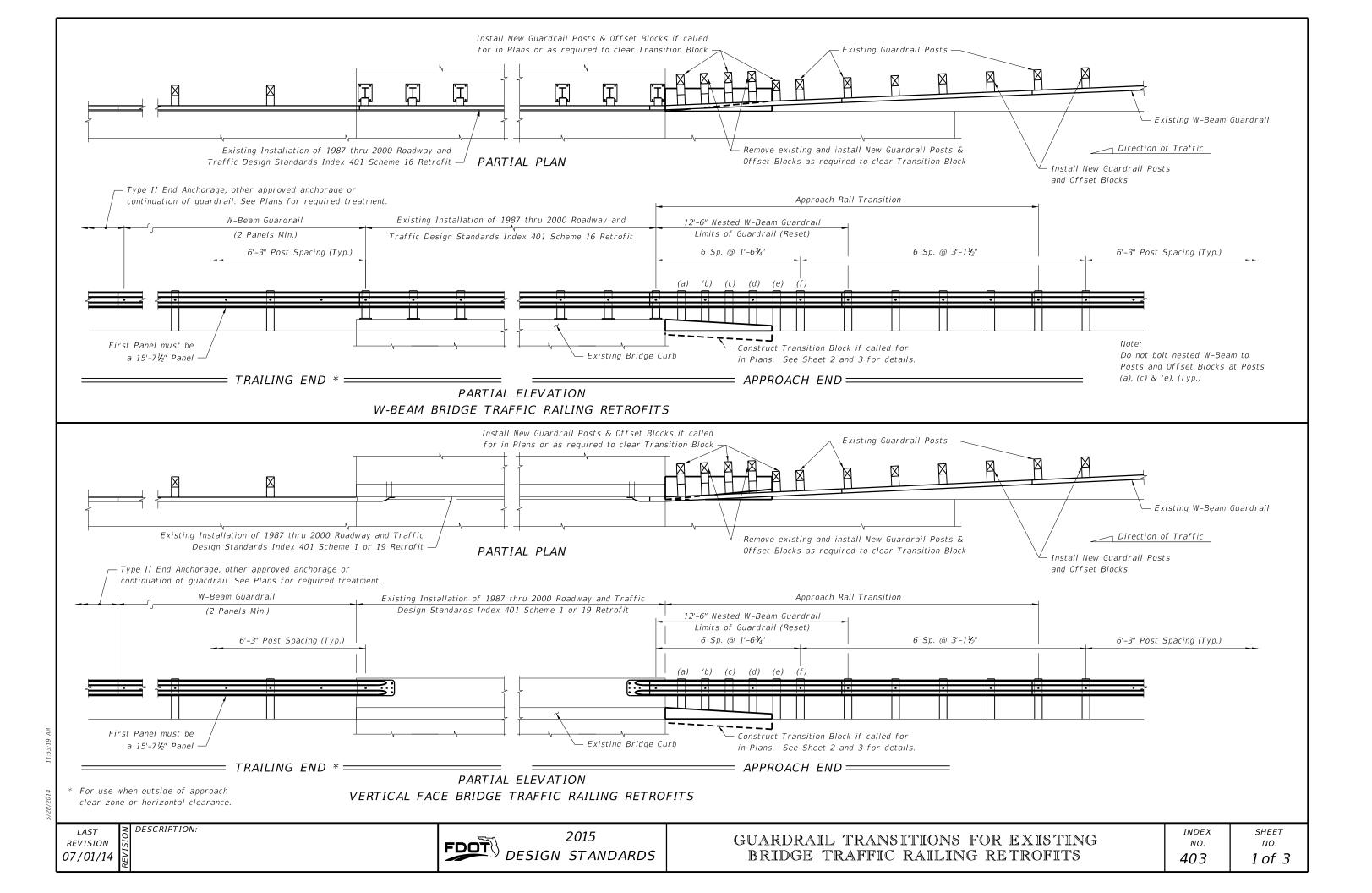


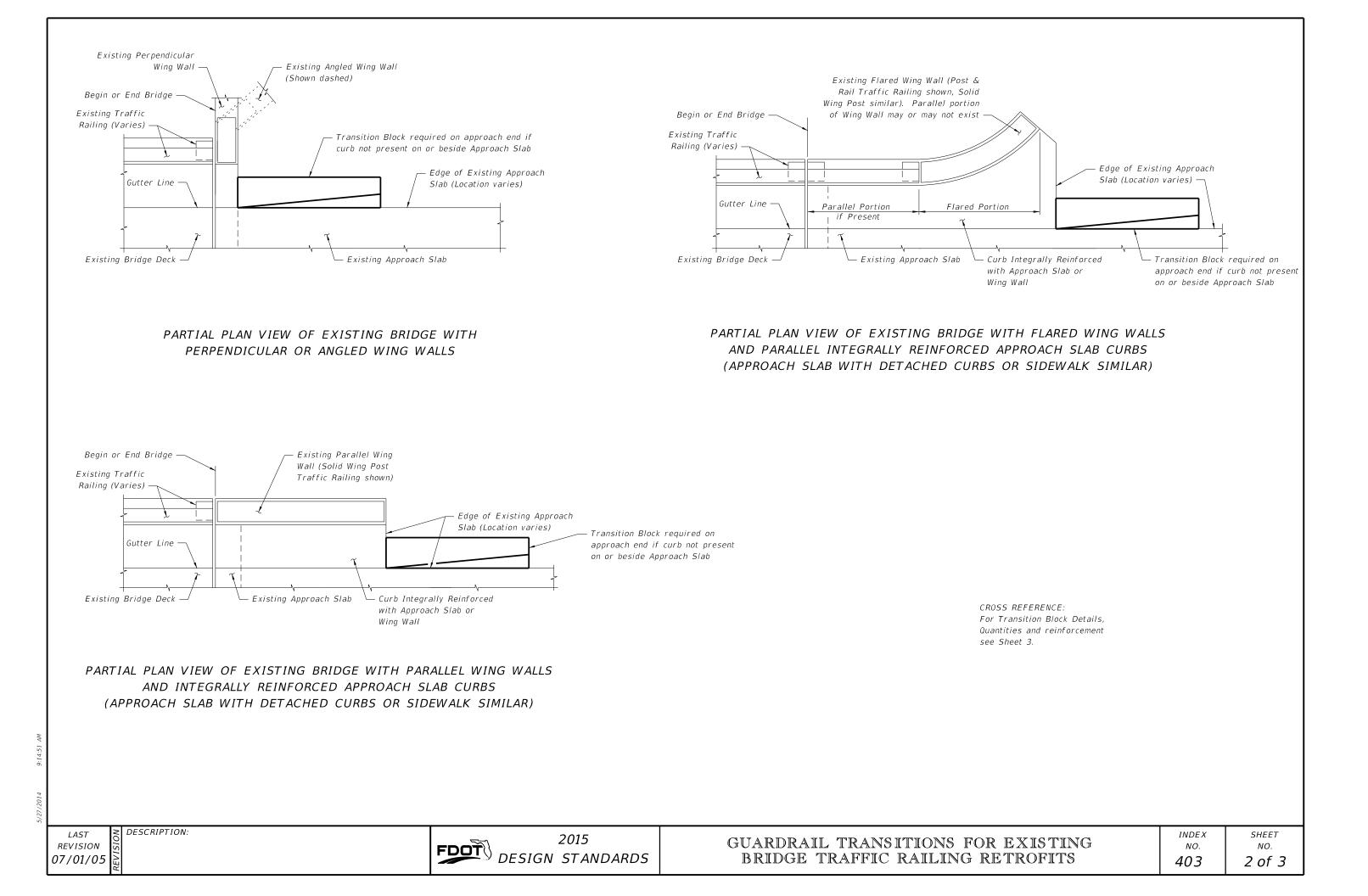
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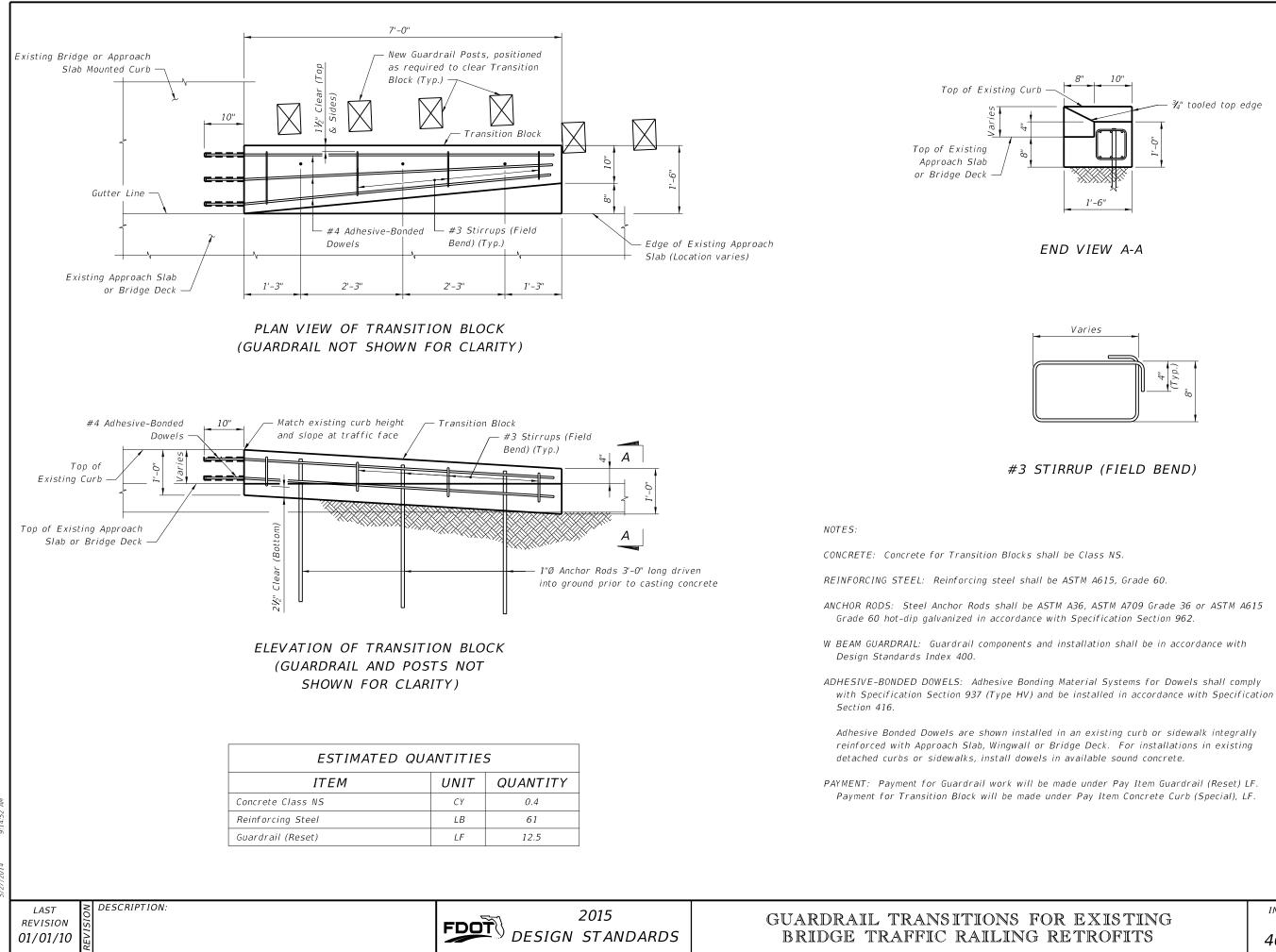












EXISTING TROFITS	INDEX NO.	SHEET NO.
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### GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

BRIDGES ON CURVED ALIGNMENTS: The details presented in this Standard are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install Barrier Delineators on top of the Traffic Railing along the entire length of the bridge 2" from the face on the traffic side at the spacing shown in the table below. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

GUARDRAIL: See Index 400 for guardrail component details, geometric layouts and associated notes not fully detailed herein.

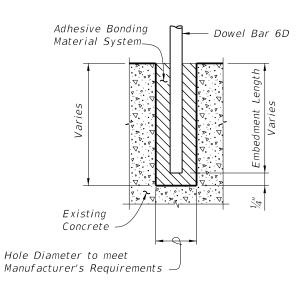
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise individual decals of letters and numbers.

PAYMENT: Guardrail Bridge Anchorage Assembly (each) includes all barrier delineators for the entire bridge length, transition blocks, and necessary hardware to complete the Guardrail transitions shown.

BARRIER DELINEATOR SPACING			
Distance – Edge of Travel Lane to Face of Railing	Spacing (Ft.)		
< 4'	40'		
4' to 8'	80'		
> than 8'	None Required		

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2015 FDOT DESIGN STANDARDS

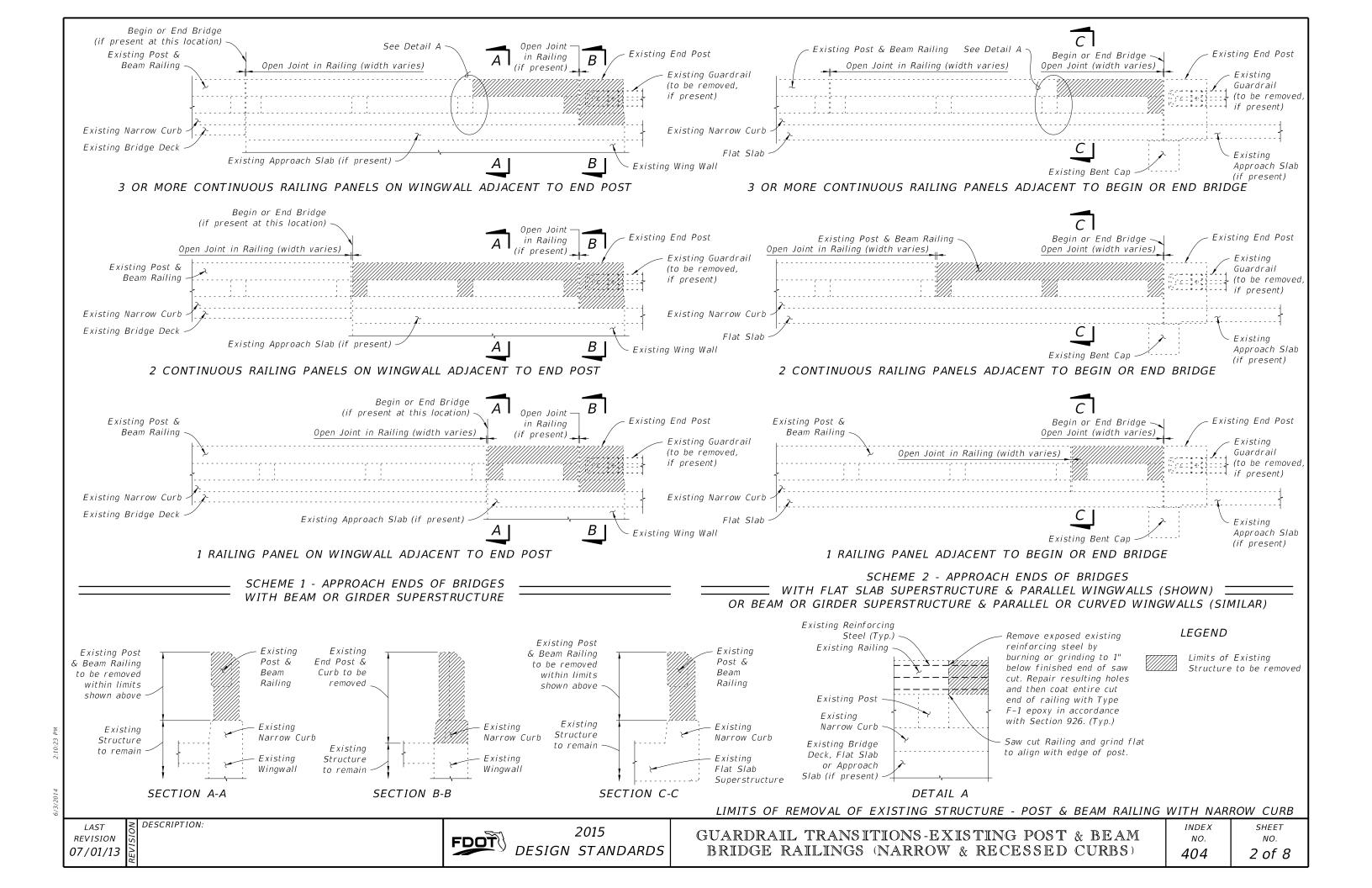


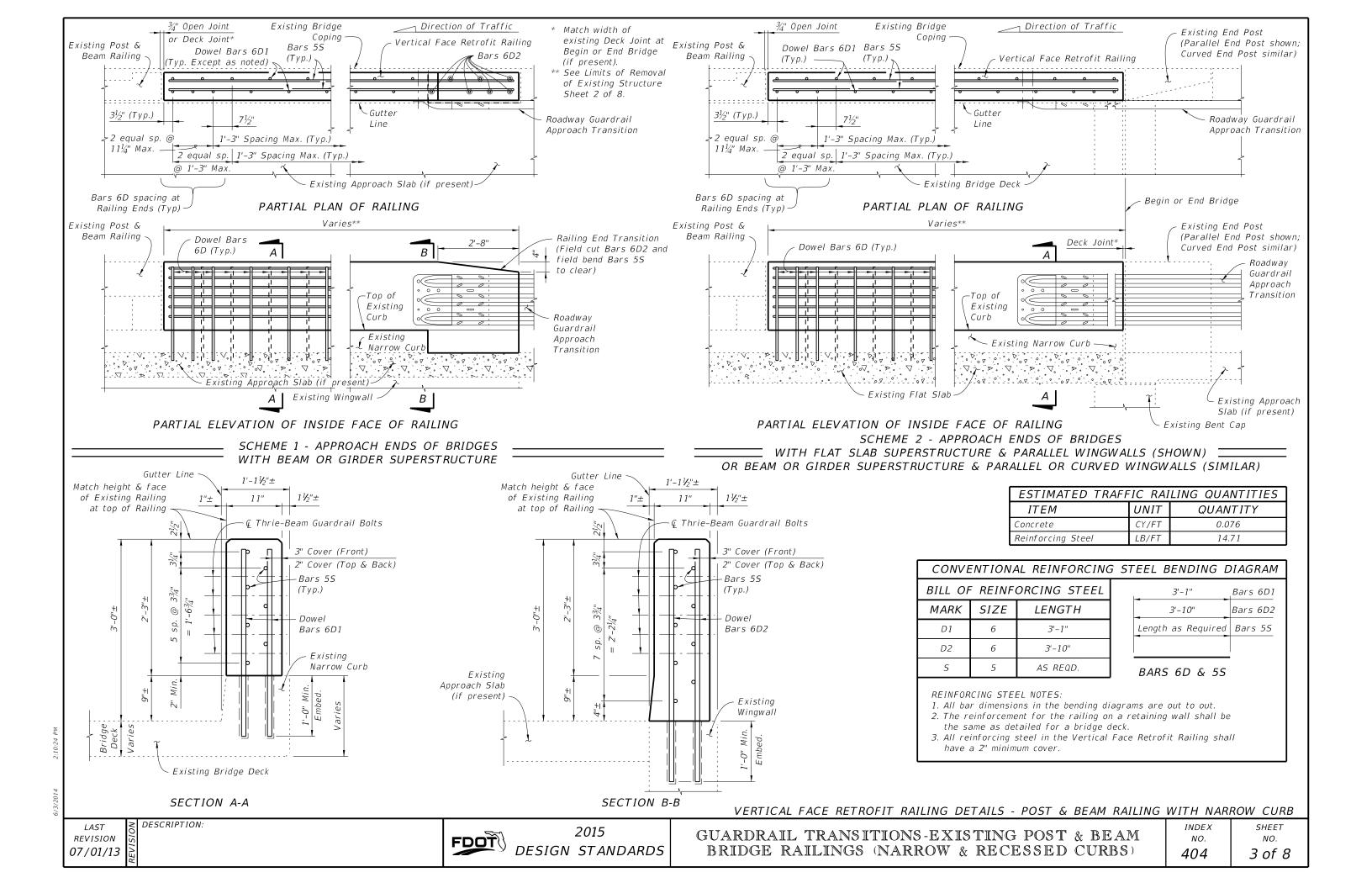
DOWEL DETAIL

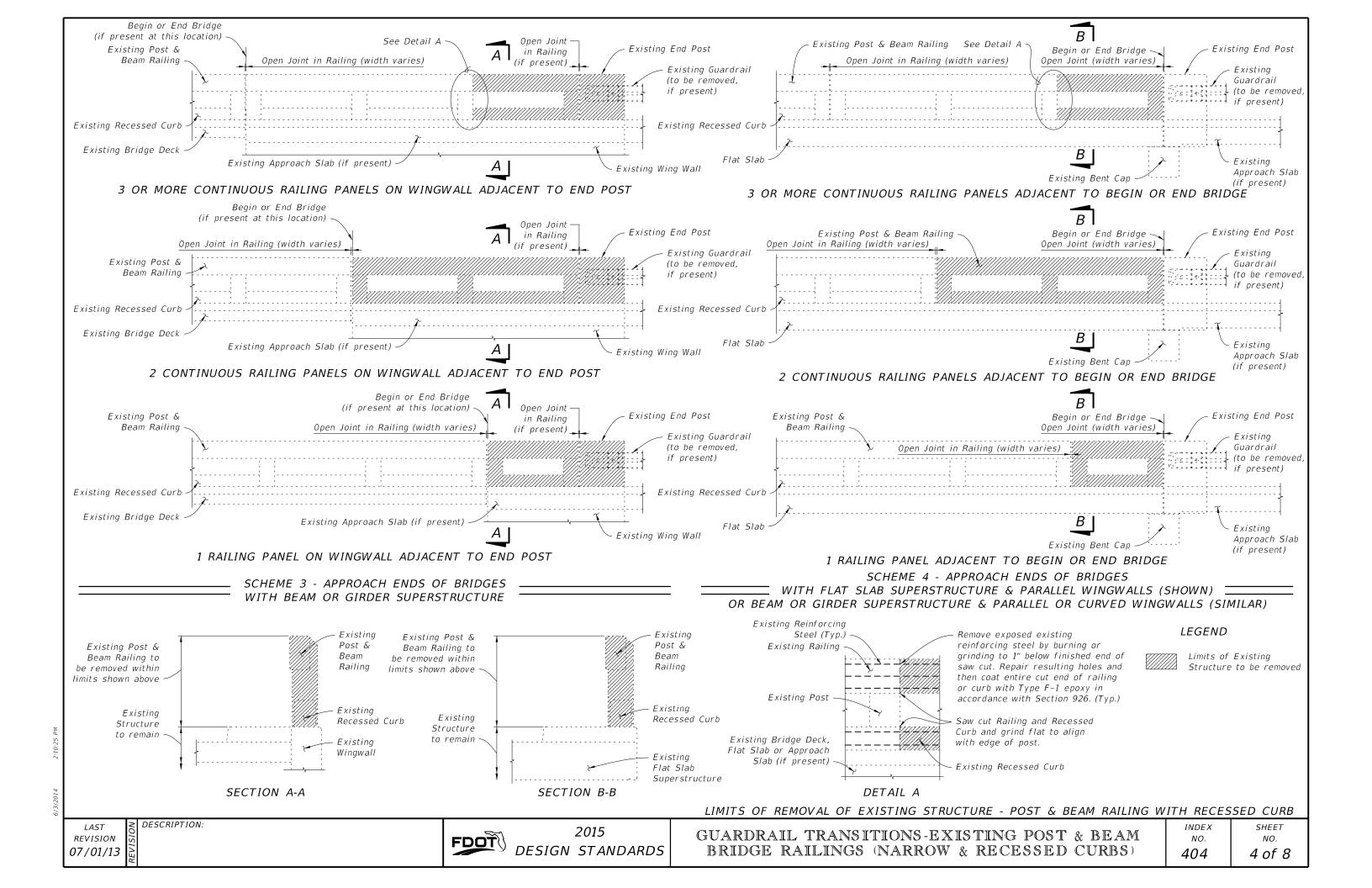
Note:

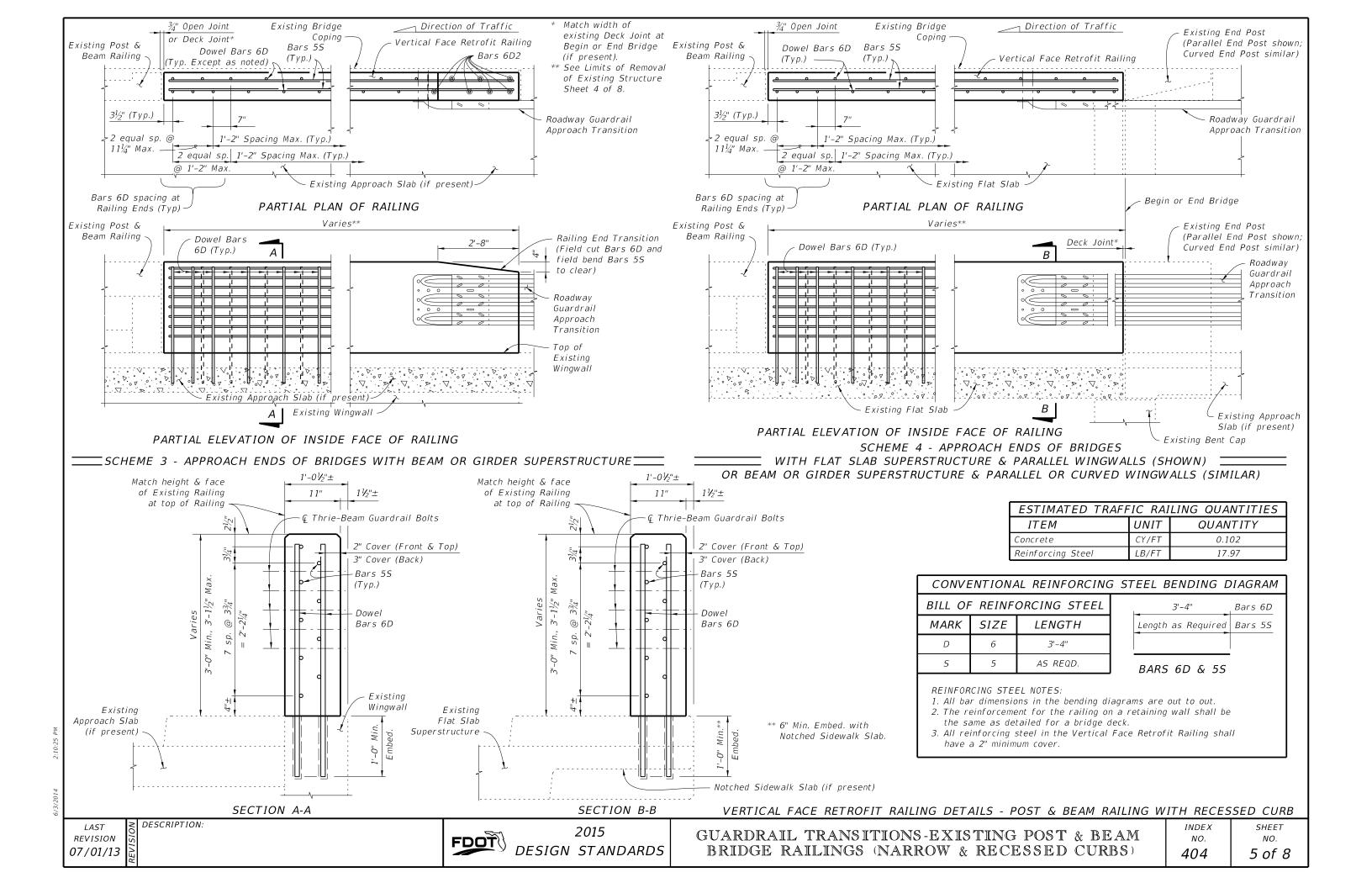
Shift dowel holes to clear if the existing reinforcement is encountered.

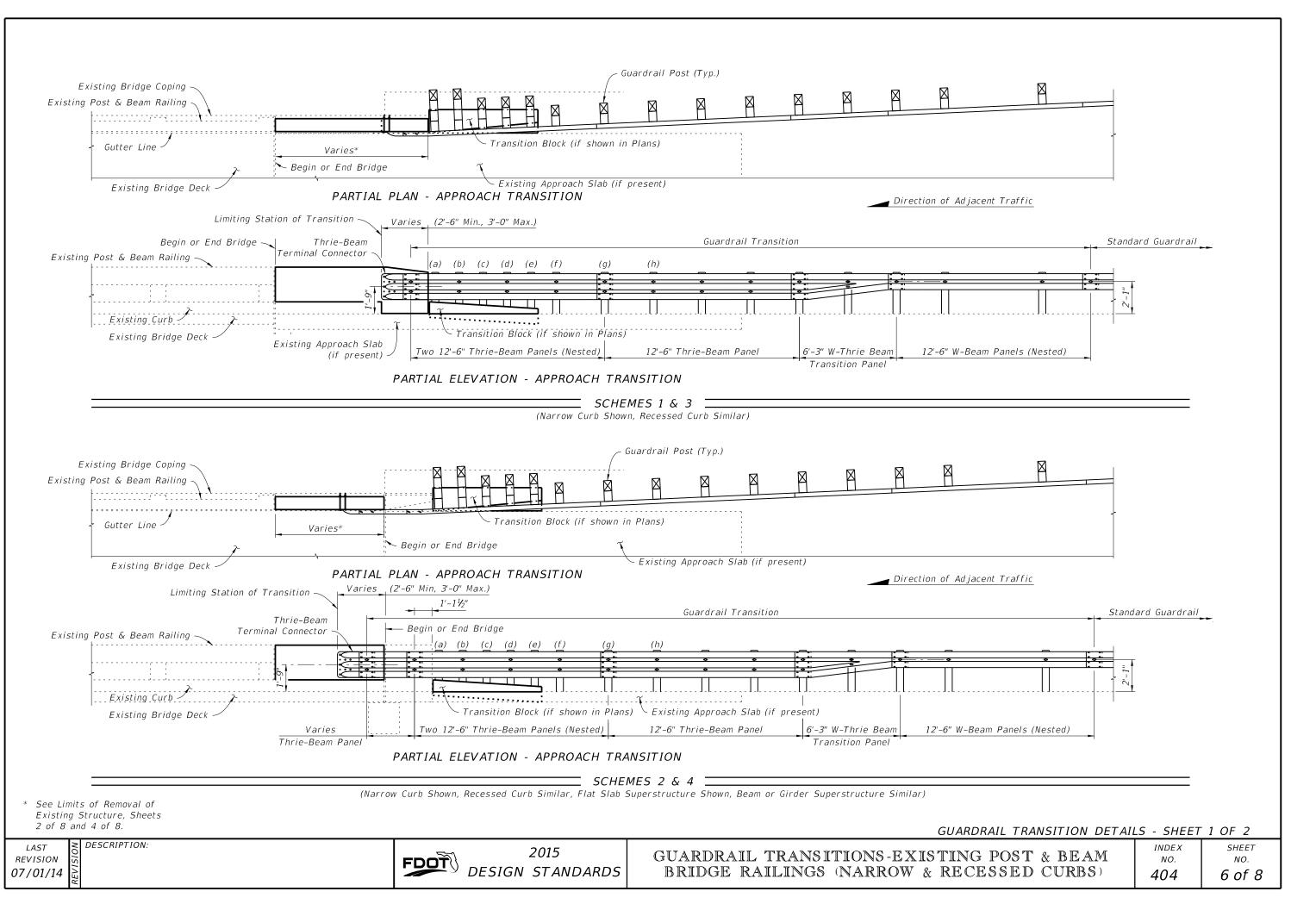
G POST & BEAM CESSED CURBS)	index no. <b>404</b>	sheet NO. <b>1 of 8</b>

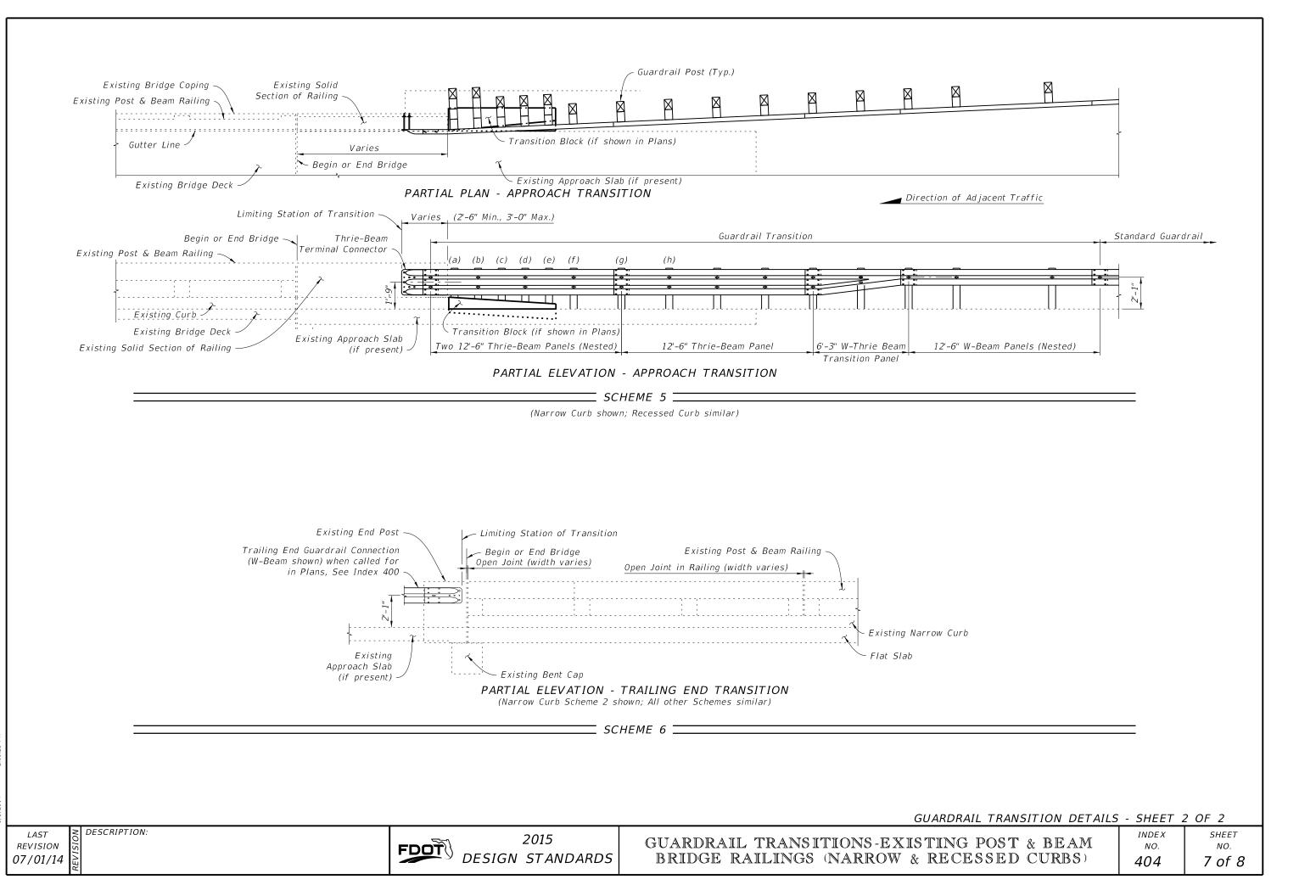


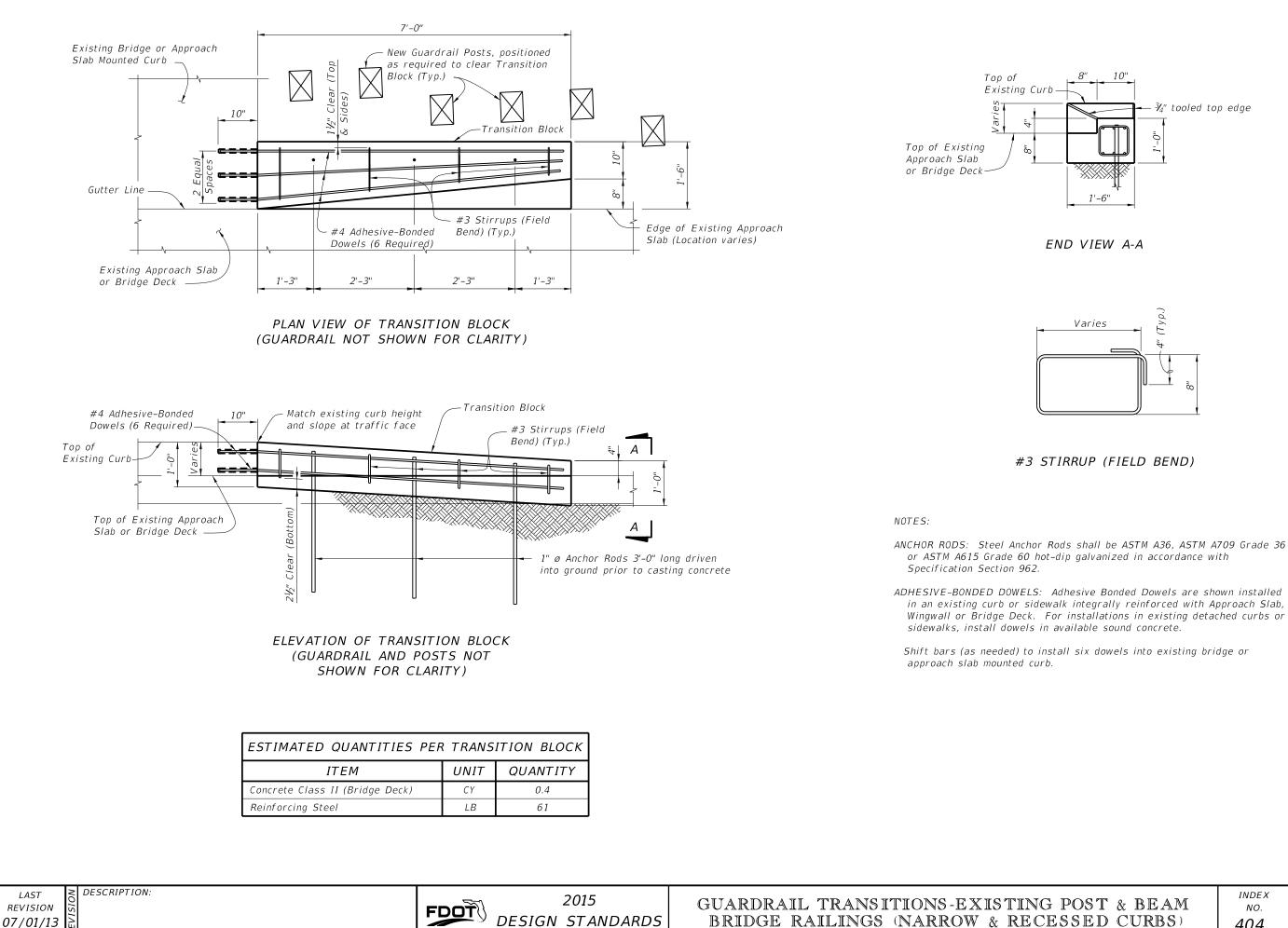












Rods	shall	be	AST	М	A36,	ASTM	A709	Grade	36
ot-dip	galva	nize	ed in	ה ה	accor	dance	with		

G POST & BEAM ESSED CURBS)	index no. <b>404</b>	<sup>SHEET</sup> NO. <b>8 of 8</b>

### GENERAL NOTES

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit) and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

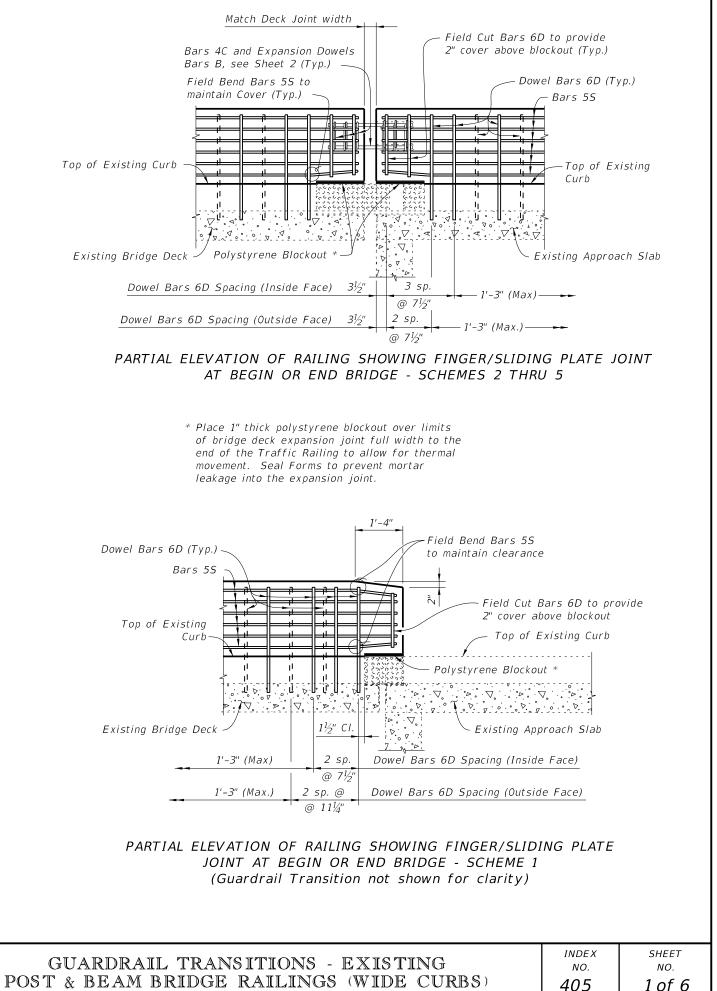
BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

BARRIER DELINEATORS: Barrier Delineators shall meet Specification Section 993. Install barrier delineators on top of the Traffic Railing along the entire length of bridge 2" from the face on the traffic side at the spacing shown in the table below. Barrier Delineator color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Concrete Traffic Railing- Bridge Retrofit- Post & Beam Railing (each) includes all materials and labor required to demolish a portion of the existing structure where required and to construct the concrete portion of the retrofit railings. Guardrail Bridge Anchorage Assembly (each) includes all barrier delineators for the entire bridge length, transition blocks, and necessary hardware to complete the Guardrail transitions shown.

	Bars 4C and Expansion Dowels Bars B, see Sheet 2 (Typ.)
	Field Bend Bars 5S to maintain Cover (Typ.)
xisting Curb	

Polystyrene Blockout \*-Existing Bridge Deck



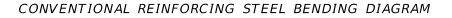
BARRIER DELINEATOR SPACING			
Distance – Edge of Travel Lane to Face of Railing	Spacing (Ft.)		
< 4'	40'		
4' to 8'	80'		
> than 8'	None Required		

ESTIMATED TRAFFIC RAILING QUANTITIES					
ITEM	UNIT	QUAN	NTITY		
11 EM	UNIT	9" Curb	Increment		
Concrete	CY/FT	0.064	0.003 per in. height		
Reinforcing Steel	LB/FT	13.27	0.10 per in. length		

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.)

DESCRIPTION: LAST REVISION 07/01/13

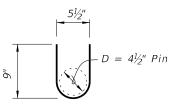
2015 FDOT DESIGN STANDARDS

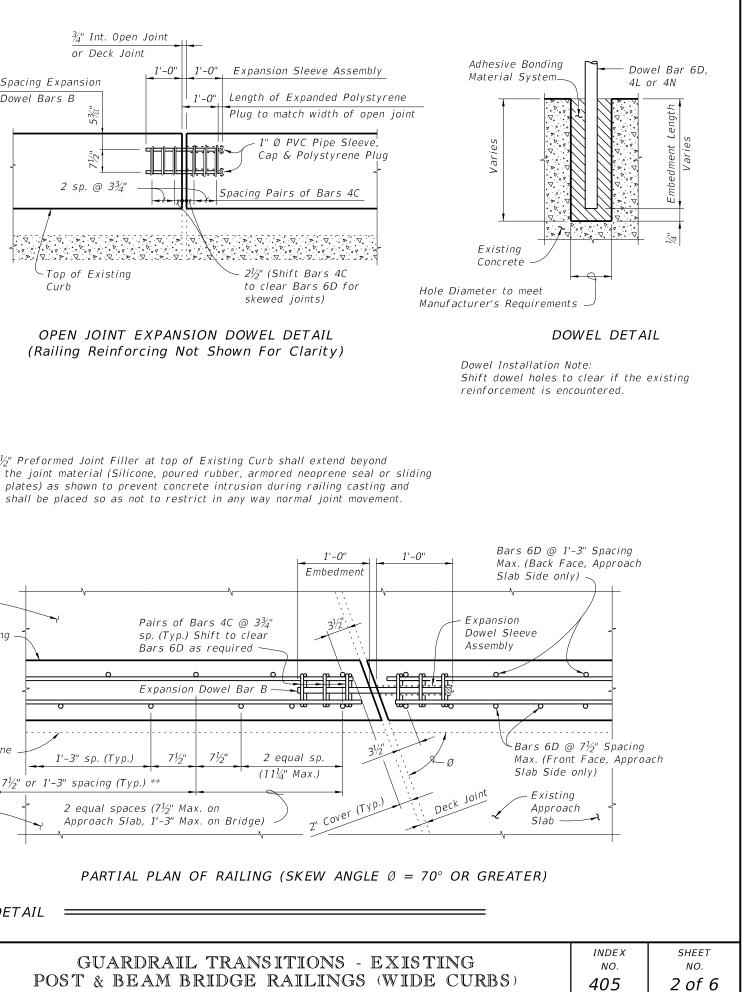


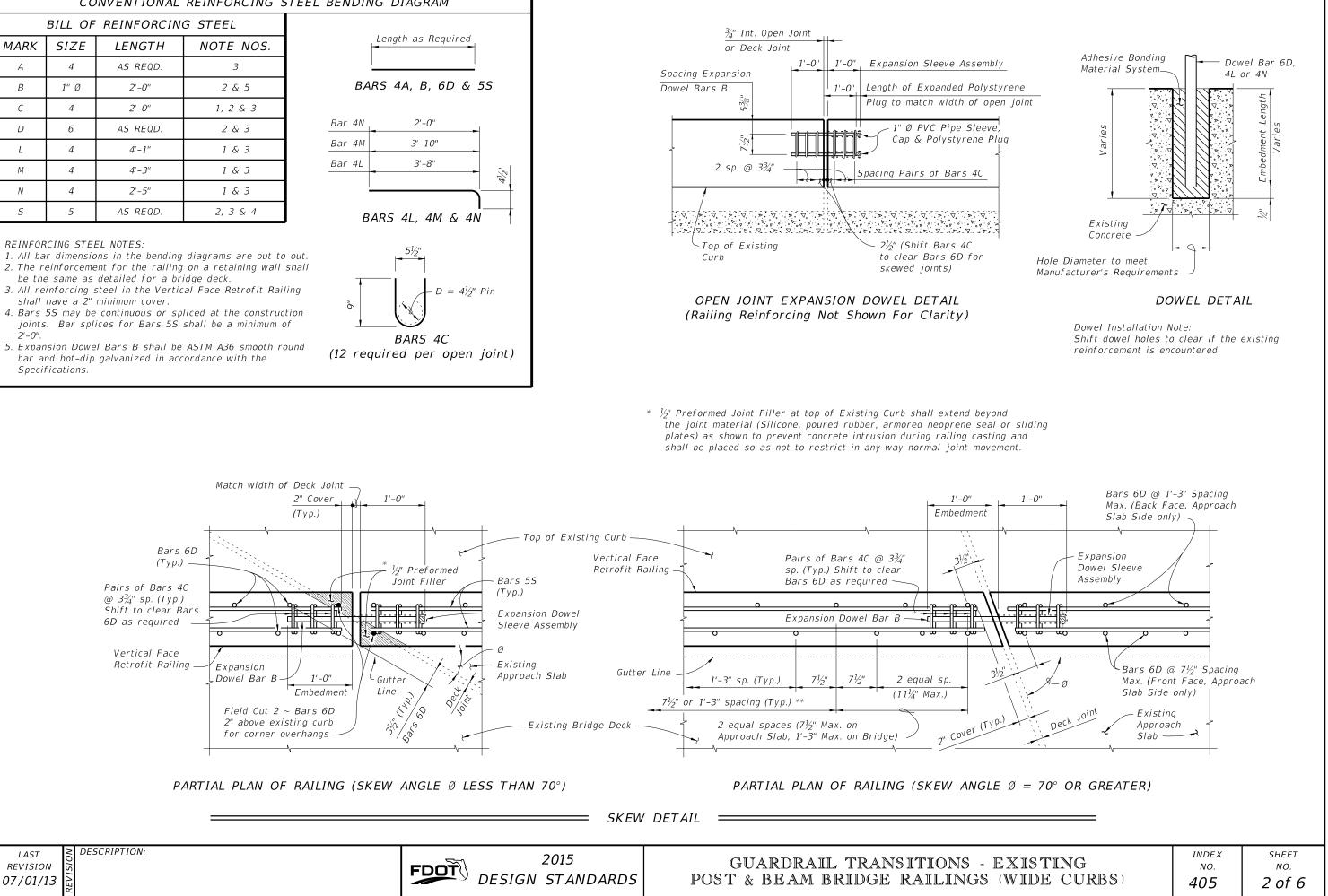
BILL OF REINFORCING STEEL

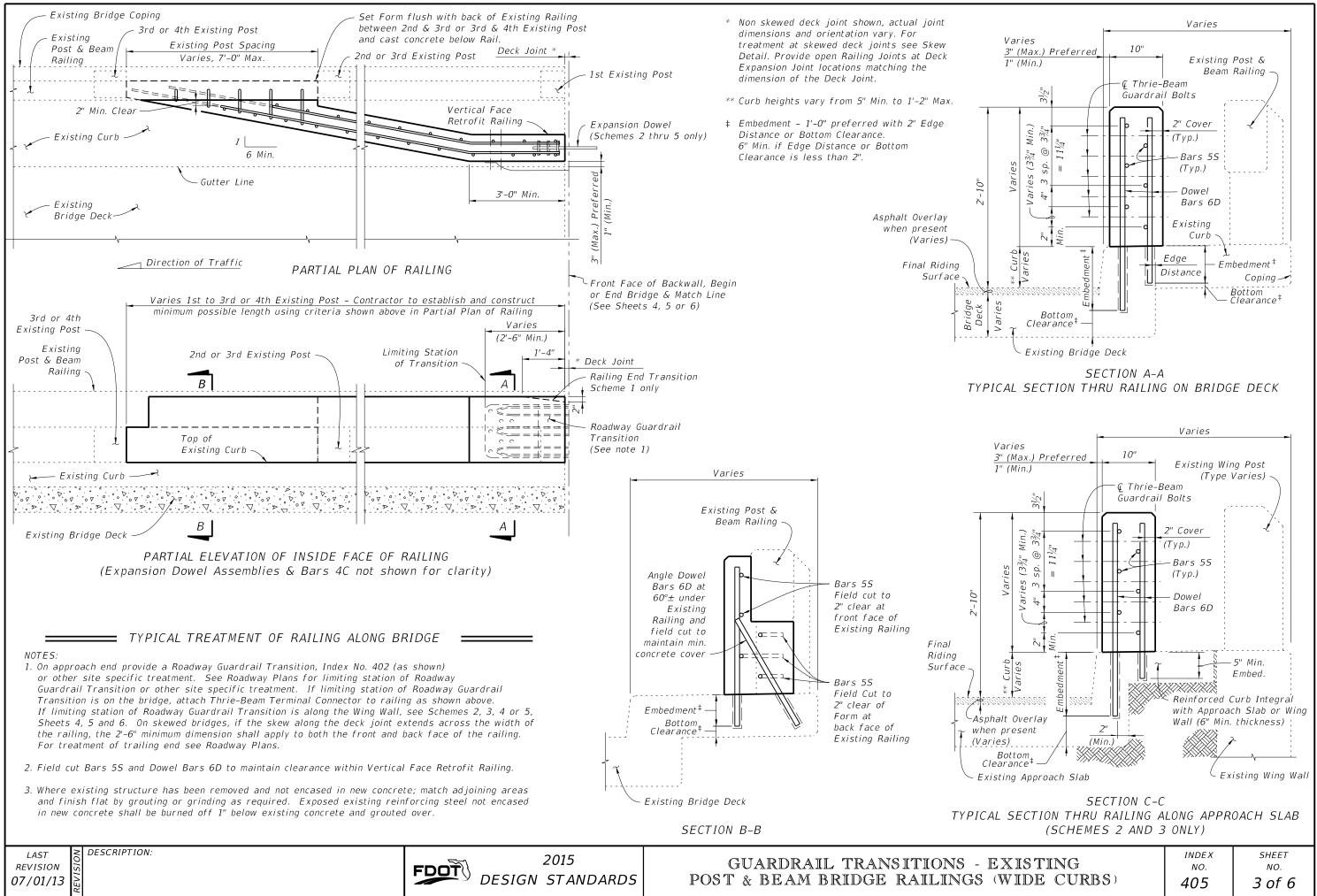
BILL OF REINFORCING STEEL					
MARK	SIZE	LENGTH	NOTE NOS.		
А	4	AS REQD.	3		
В	1" Ø	2'-0''	2 & 5		
С	4	2'-0''	1, 2 & 3		
D	6	AS REQD.	2 & 3		
L	4	4'-1''	1 & 3		
М	4	4'-3"	1 & 3		
N	4	2'-5"	1 & 3		
5	5	AS REQD.	2,3&4		

Length as Required BARS 4A, B, 6D & 5S Bar 4N 2'-0" 3'-10" Bar 4M

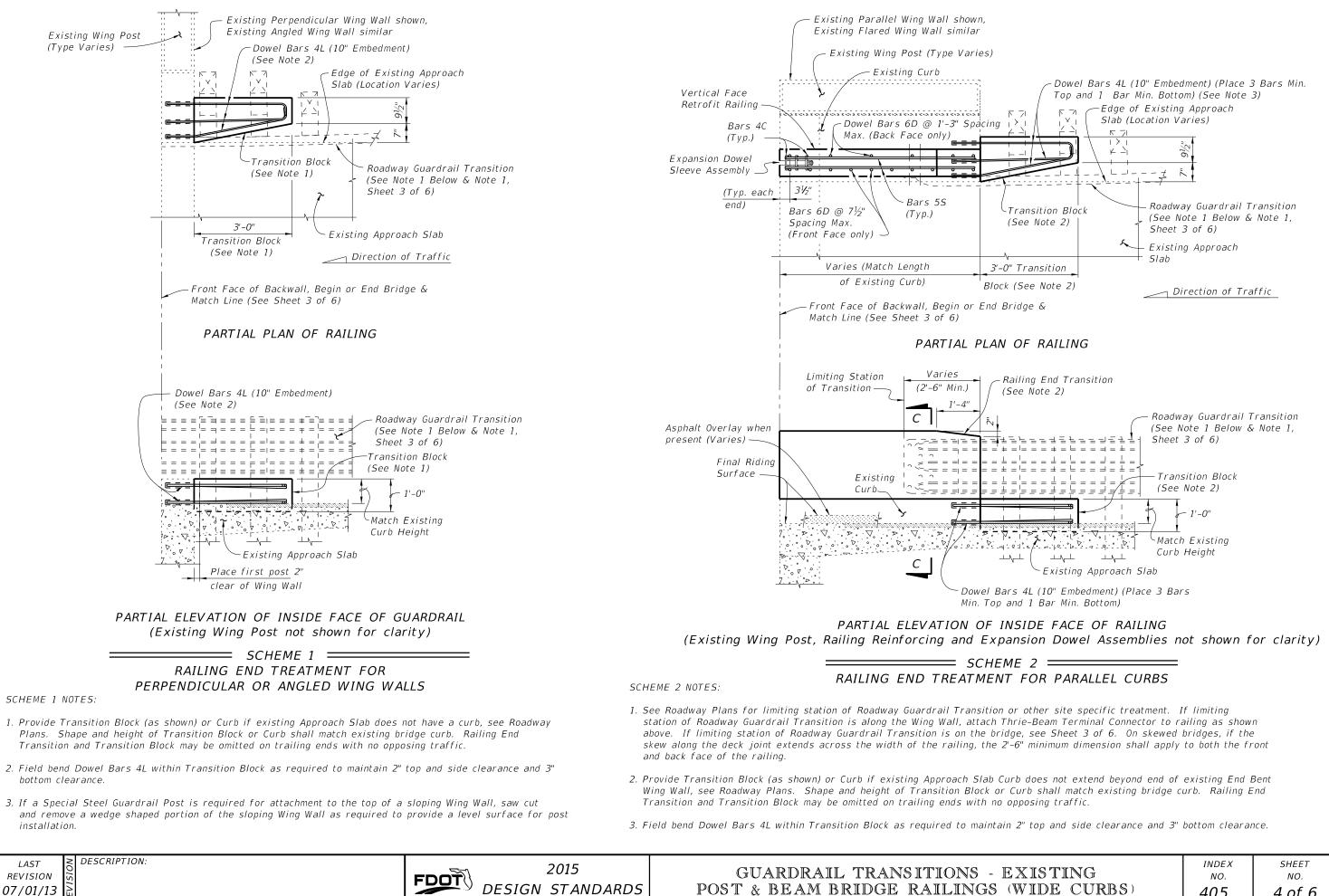




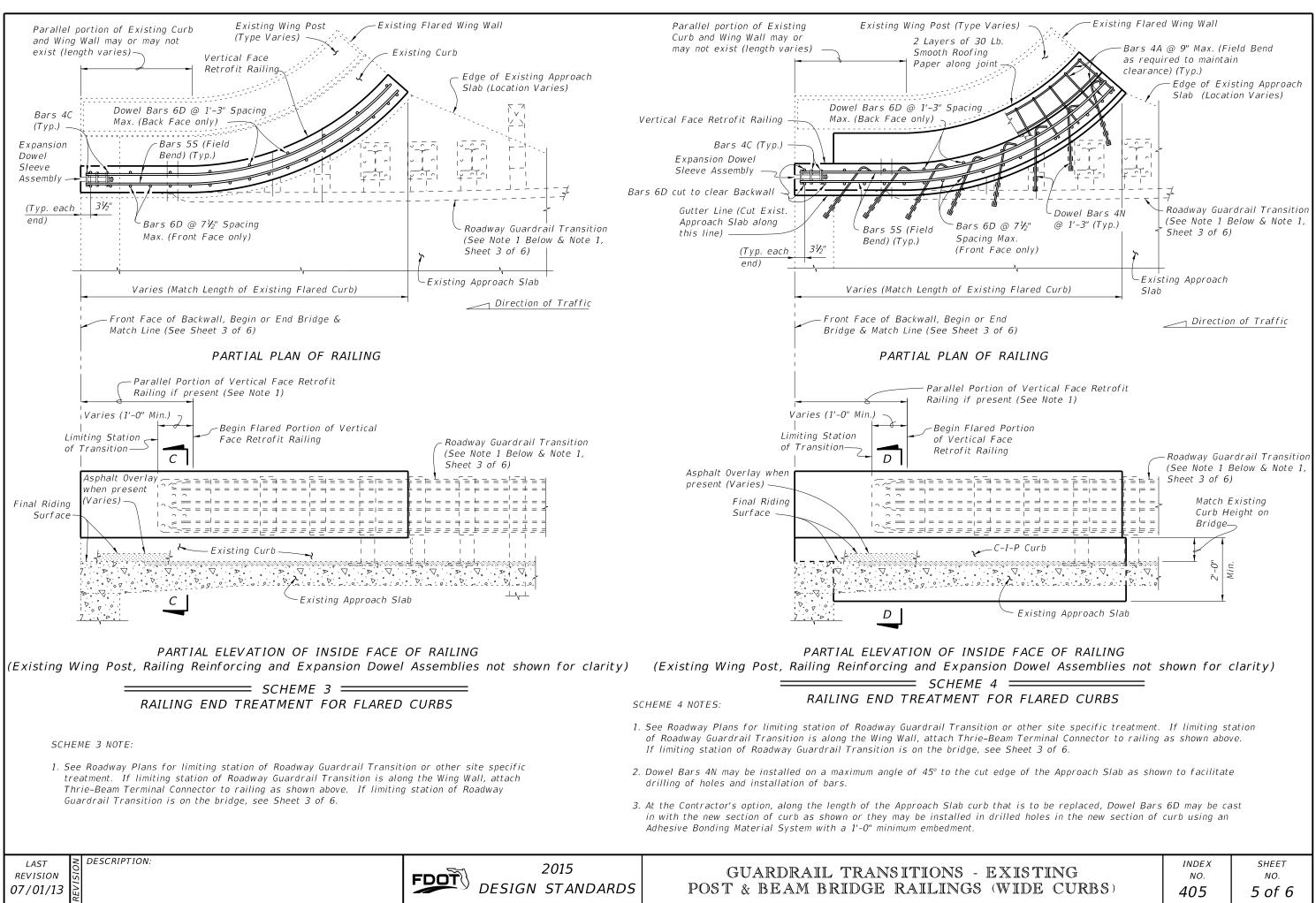




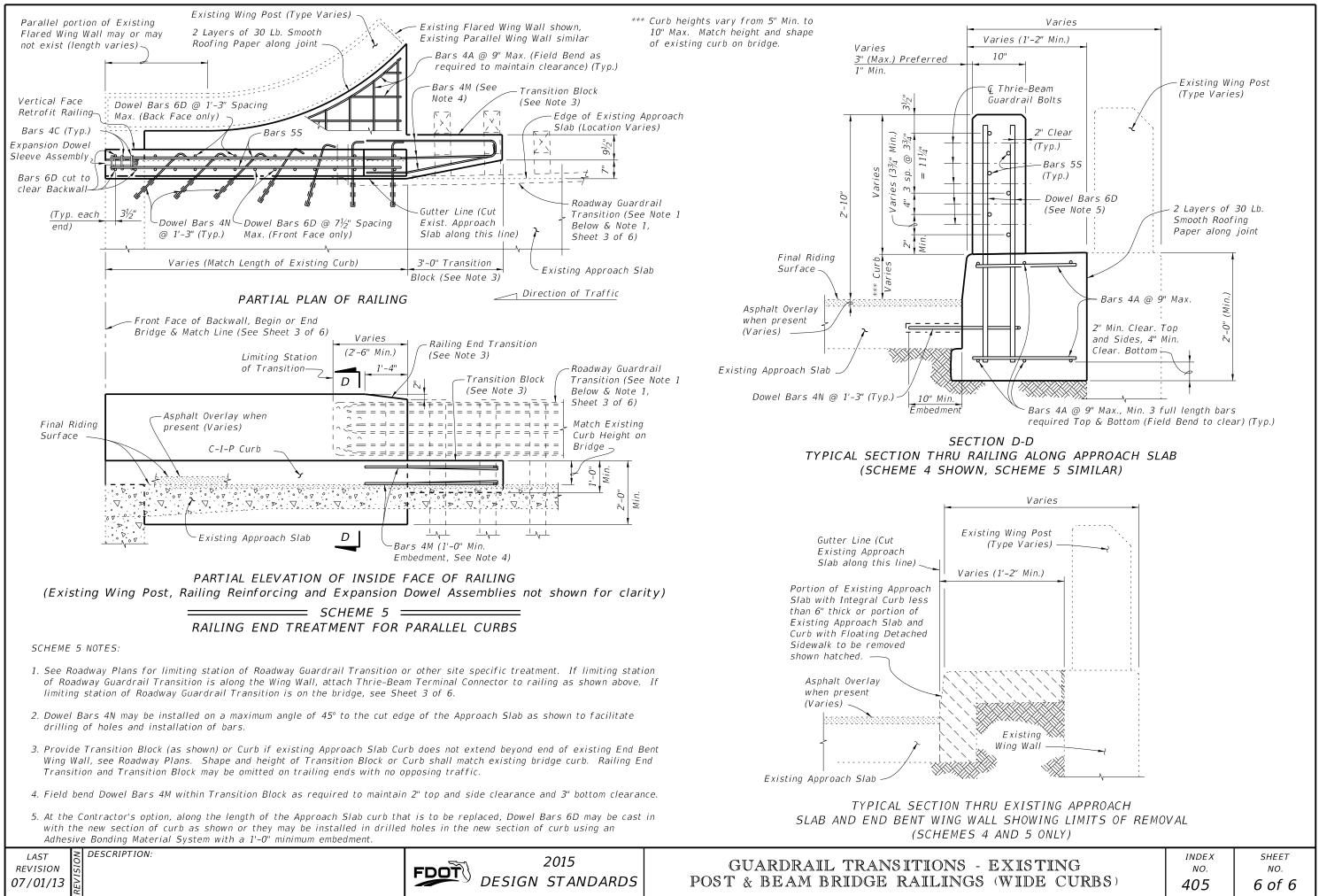
5/29/20



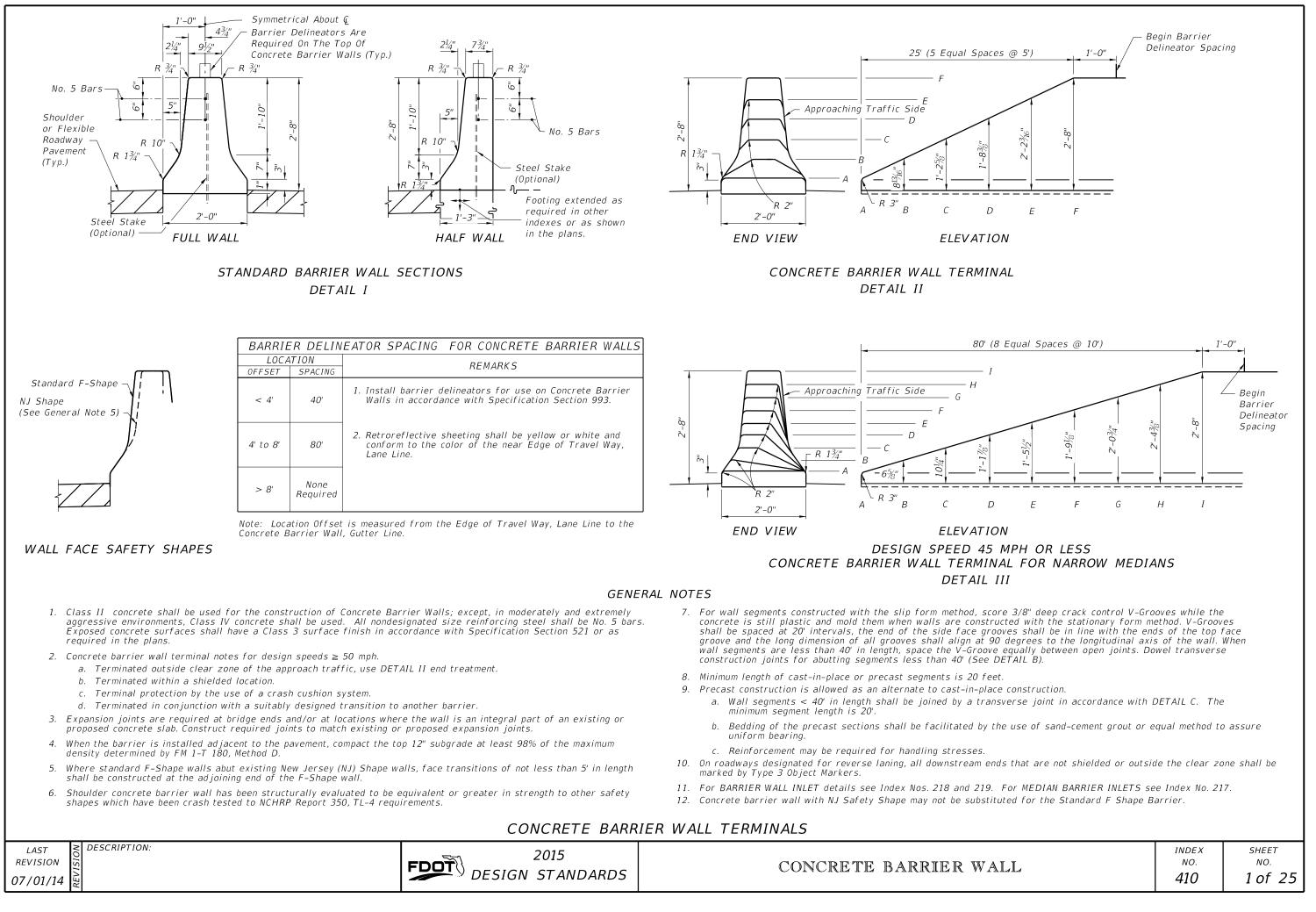
XISTING	INDEX NO.	SHEET NO.
WIDE CURBS)	405	4 of 6

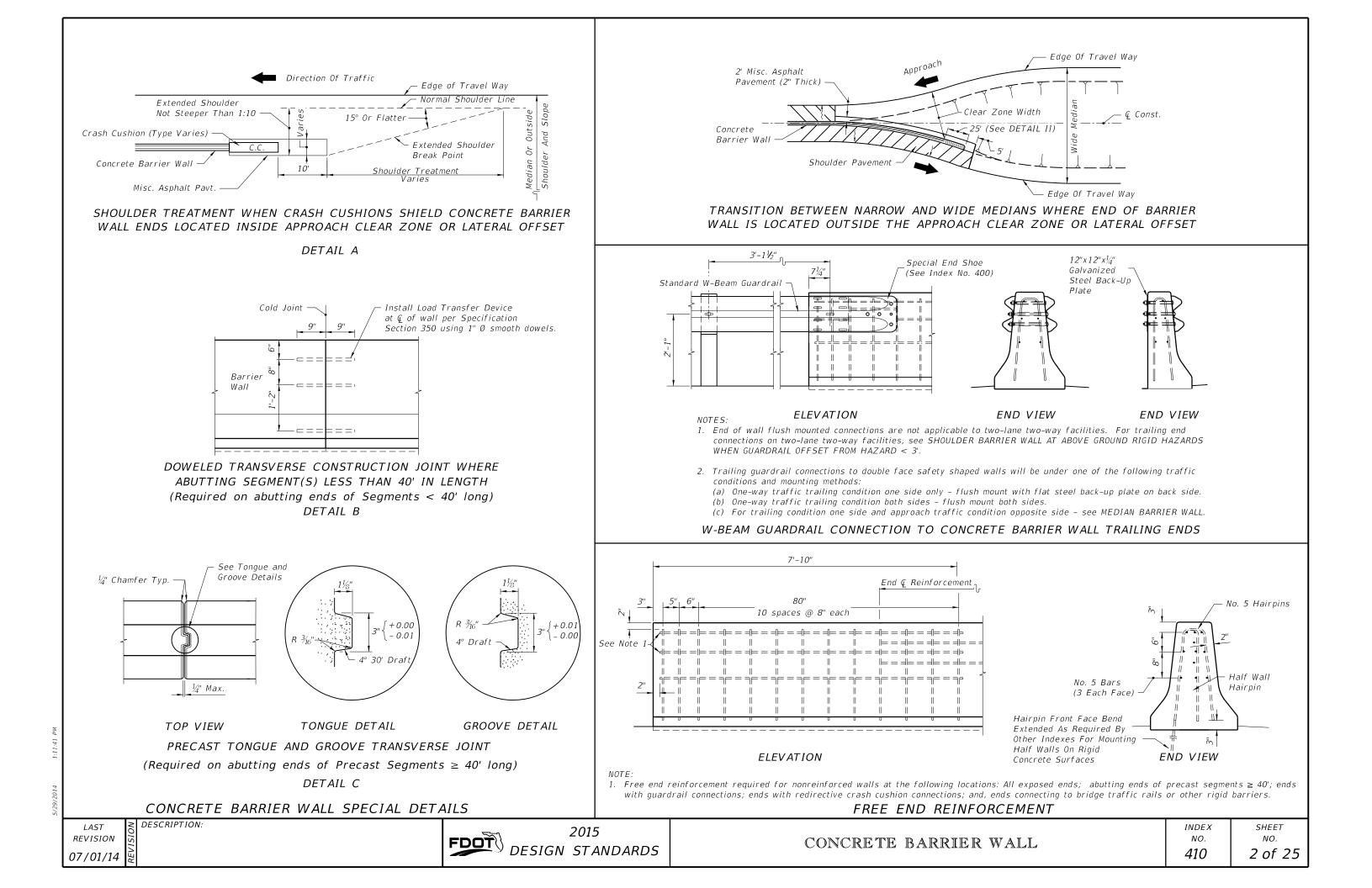


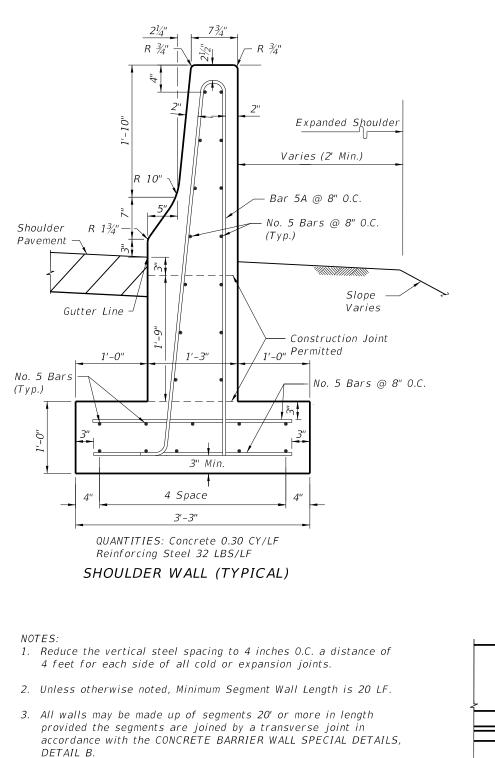
5/29/2014 1



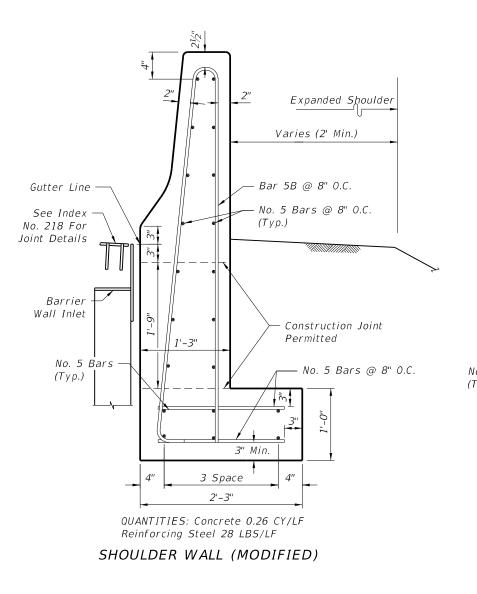
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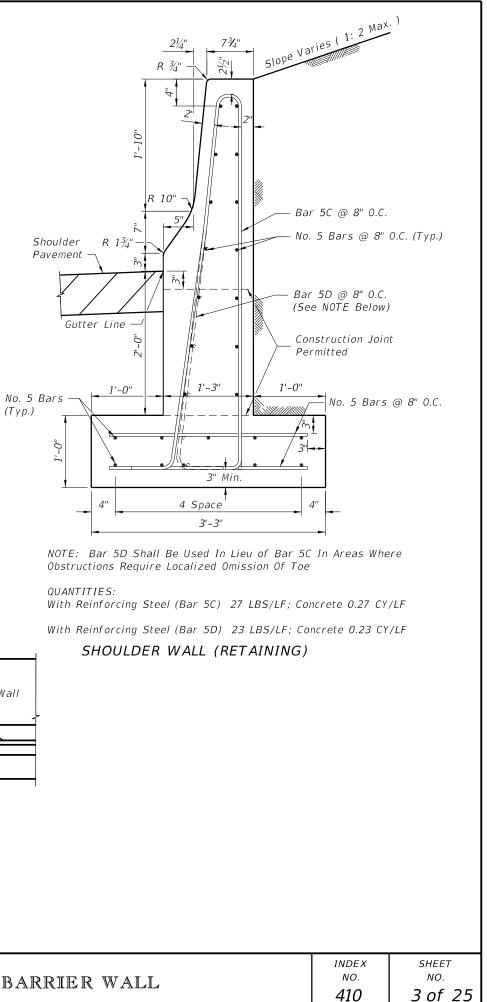


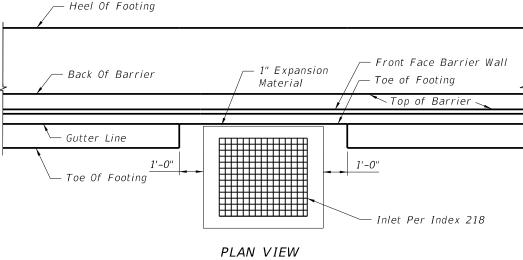




- 4. Quantities shown are for information only. Barrier wall inlets (Index 218) shall be isolated from the barrier wall stem and footing by 1" expansion material.
- 5. All longitudinal reinforcement are No. 5 bars.
- 6. For additional information on Bars 5A, 5B, 5C and 5D, see BAR BENDING DIAGRAMS.







# REINFORCED CONCRETE SHOULDER WALL

LAST	NC	DESCRIPTION:
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2015 DESIGN STANDARDS

CONCRETE BARRIER WALL



MEDIAN BARRIER WALL FOR SUPERELEVATED SECTIONS WITH VARIABLE ROADWAY PROFILE GRADE LINES

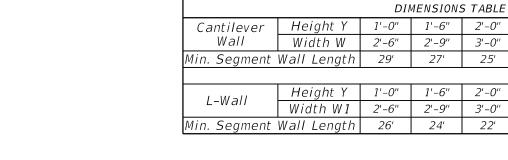
CONCRETE	BARRIER	WAL

		2015
FDOT	DESIGN	STANDARDS

MEDIAN	L

LAST REVISION	SION	DESCRIPTION:

07/01/14 🛓



NO	ΤE
1.	UI
	С
	С
2	_
2.	F

	F-SHAI	PE MEDIA	N BARRIE	R
WHEN Y	IS LESS	THAN OF	R EQUAL 1	TO 6 INCHES

# CANTILEVER WALL

2'-0''

3'-0''

25'

2'-0''

3'-0''

22'

2'-6"

3'-3''

23'

2'-6"

3'-3"

21'

3'-0''

3'-3"

24'

3'-0"

3'-3"

22'

3'-6"

3'-6"

22'

3'-6"

3'-6"

21'

4'-0''

3'-6"

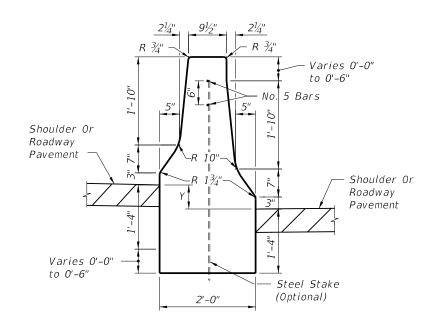
24'

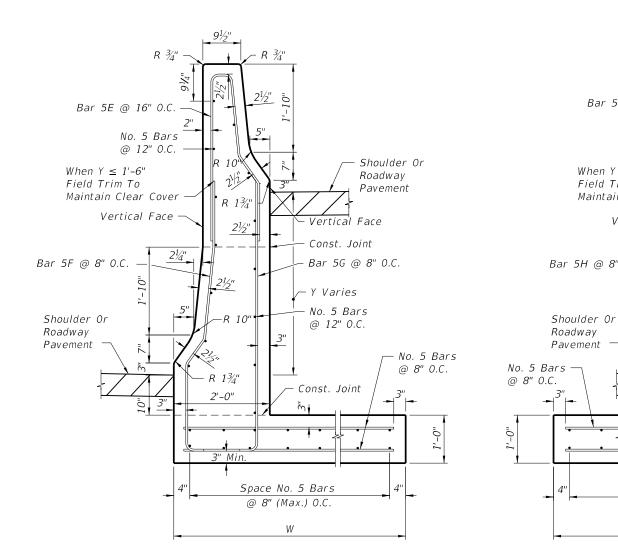
4'-0''

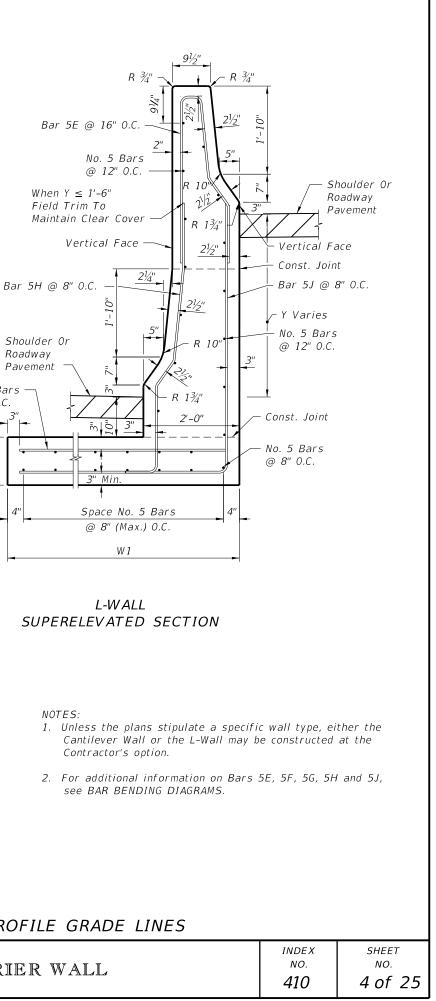
3'-6"

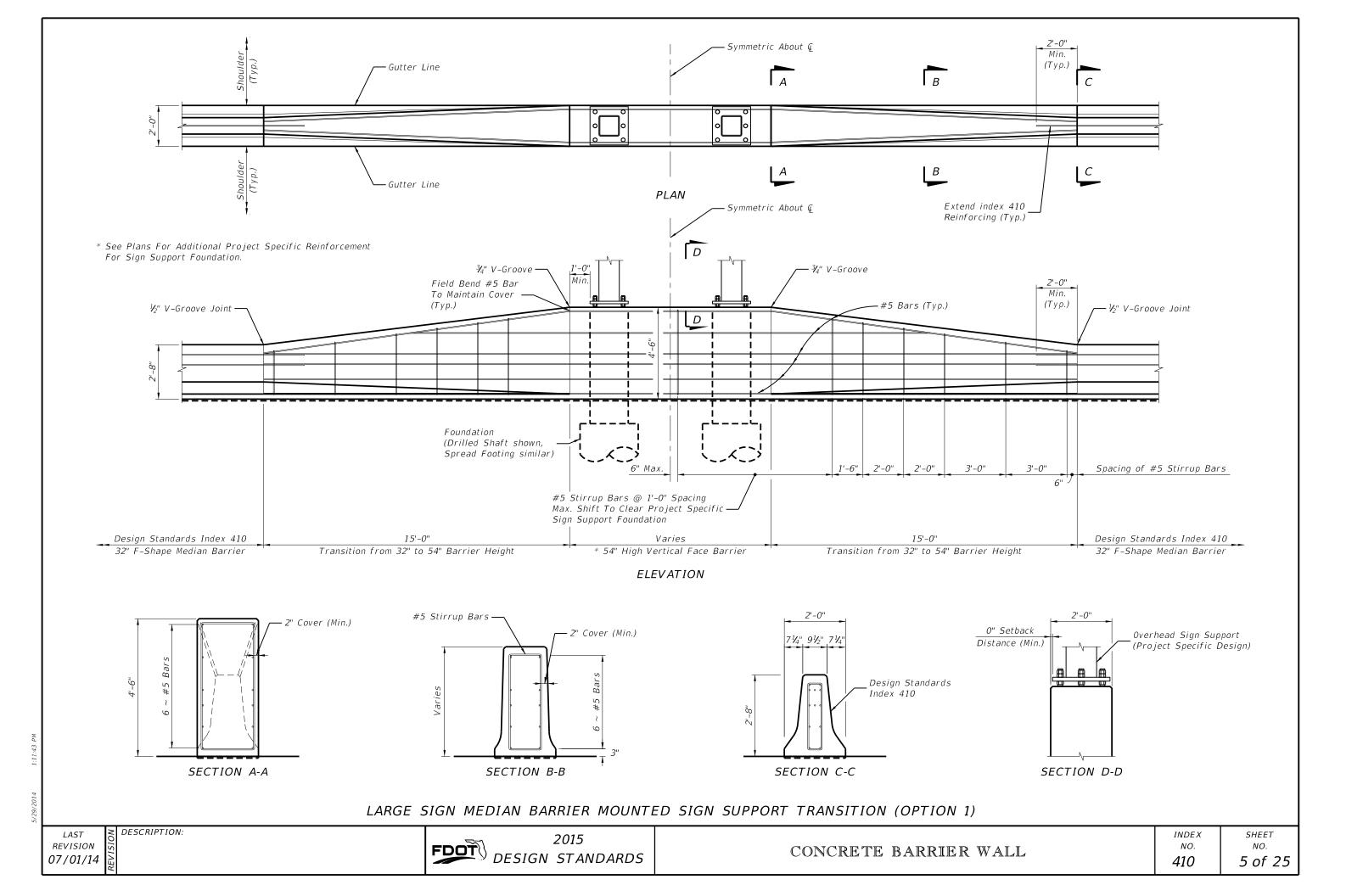
24'

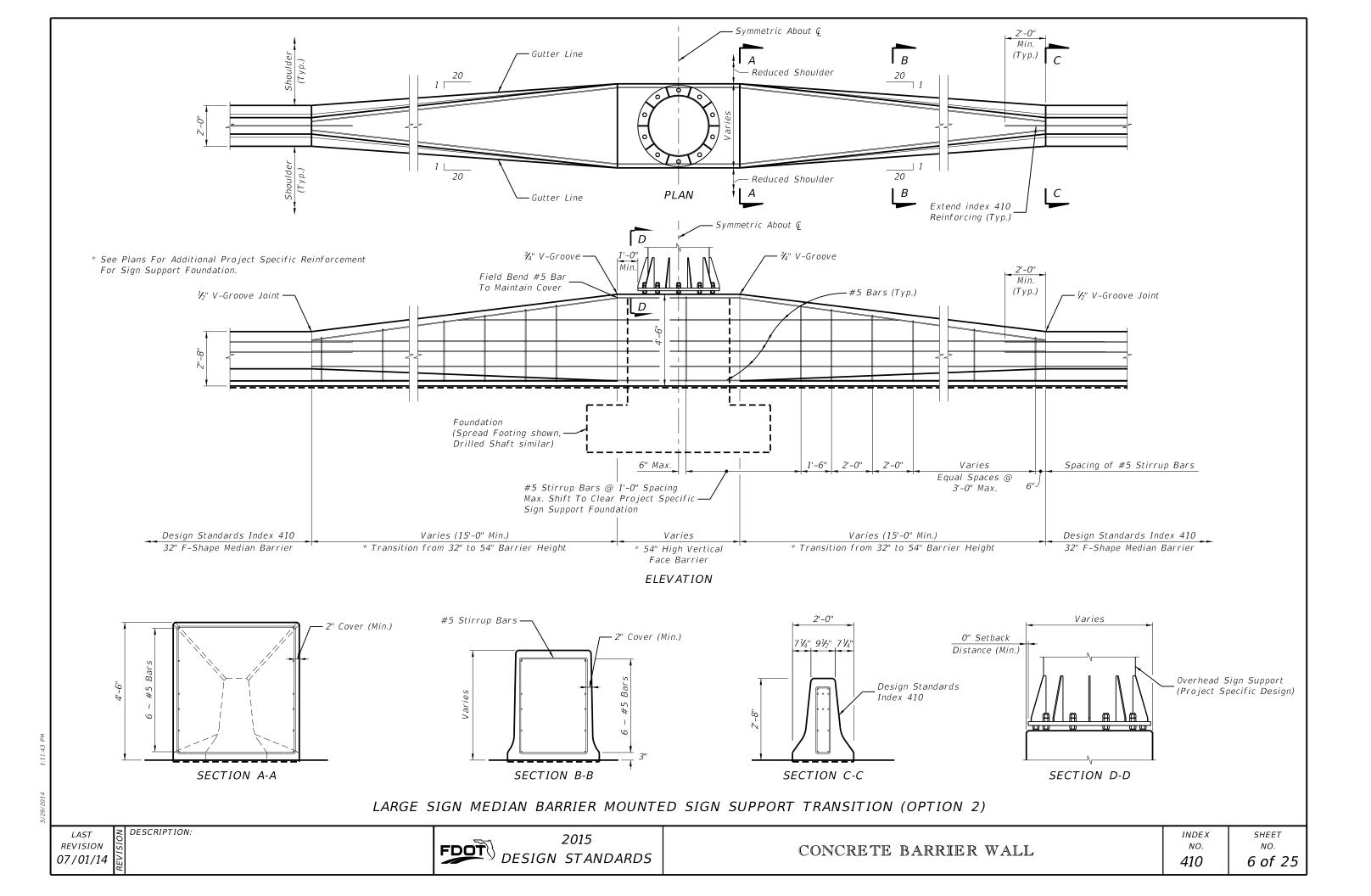
# SUPERELEVATED SECTION

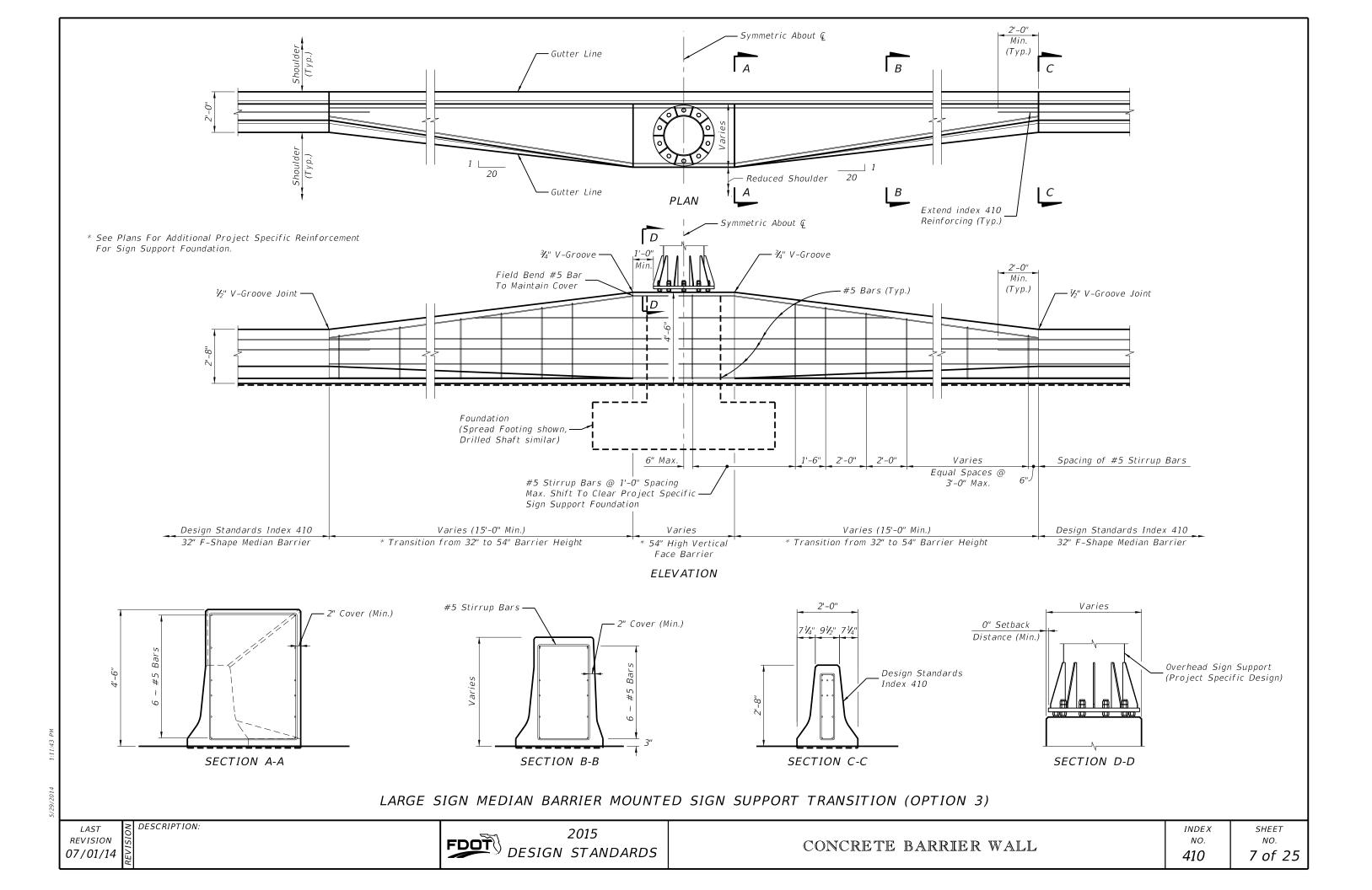


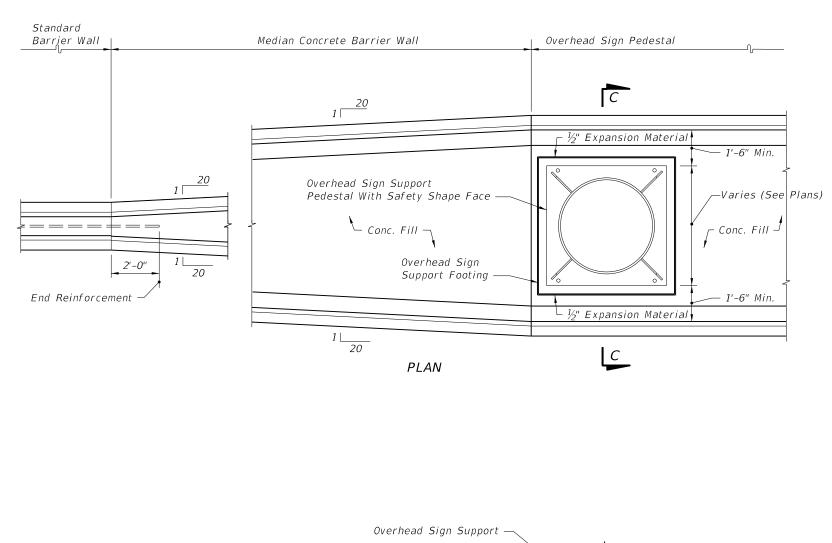


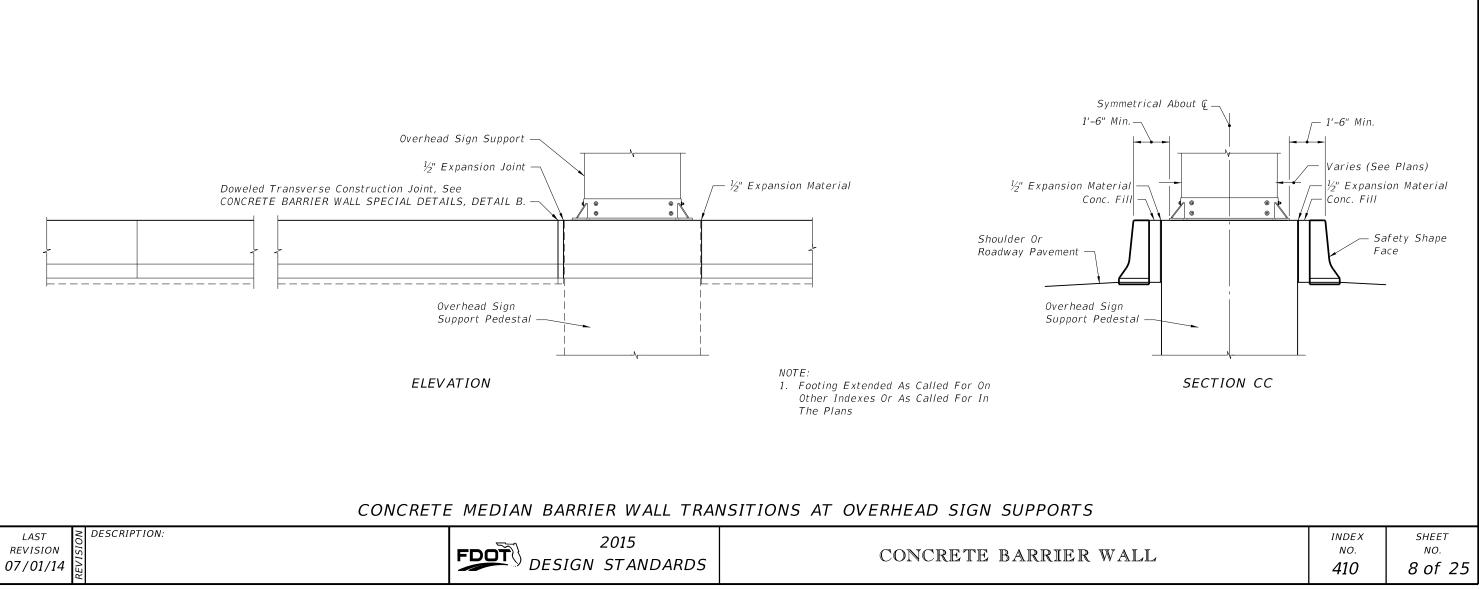




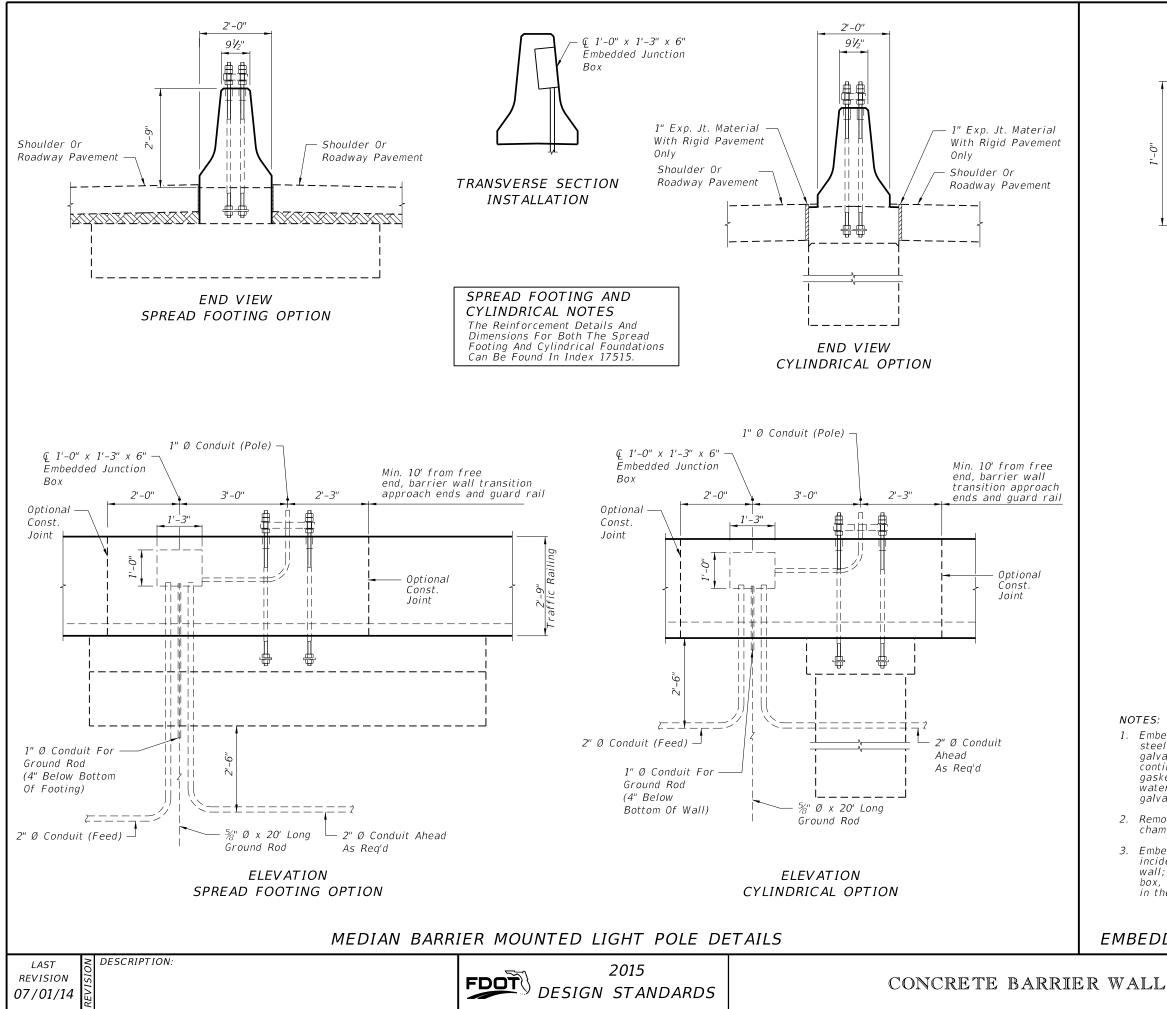


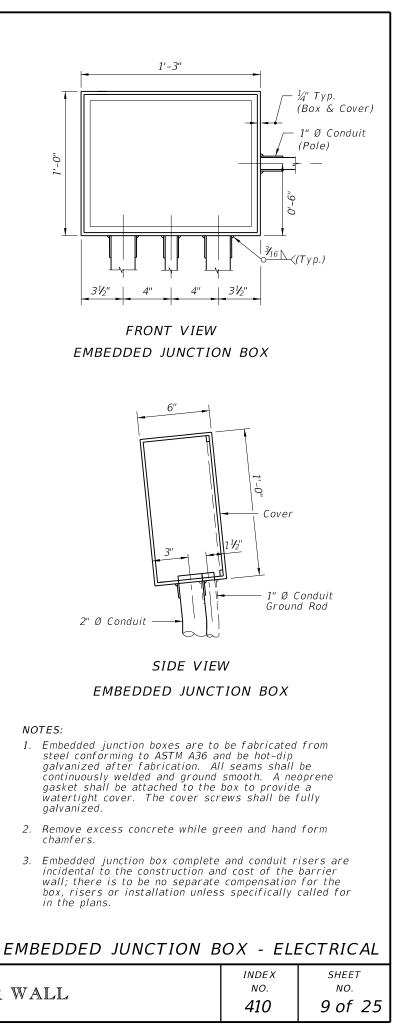


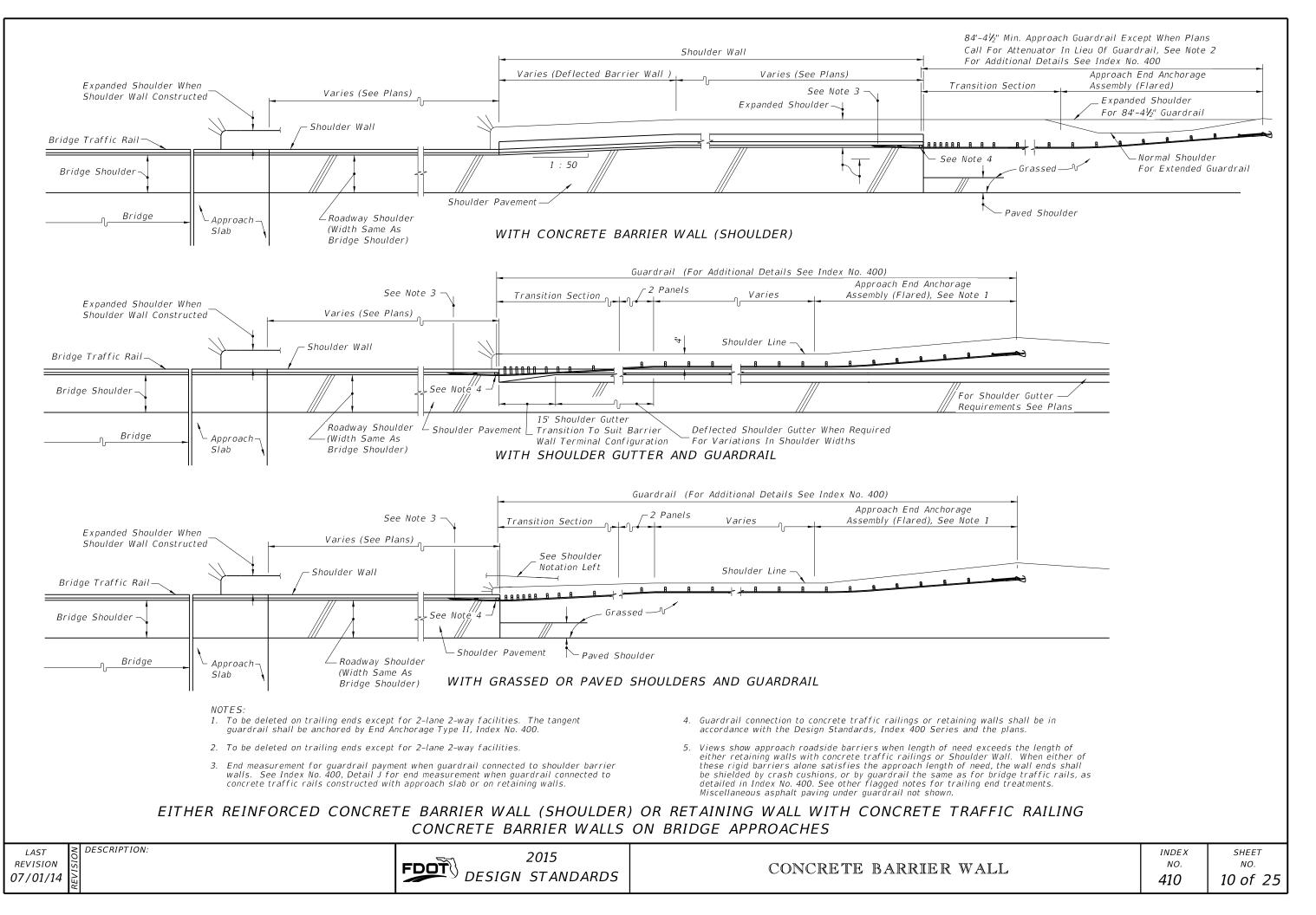


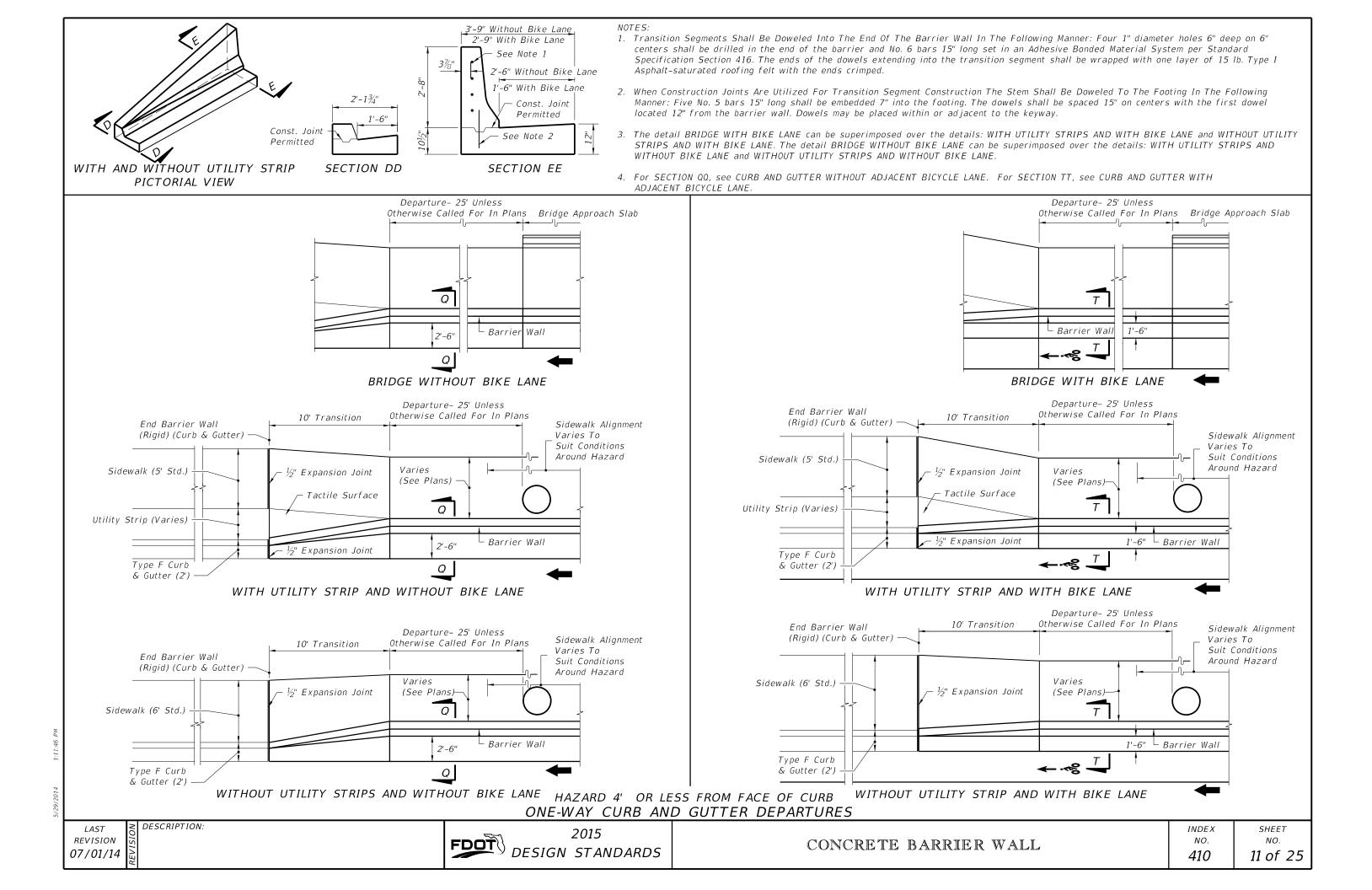


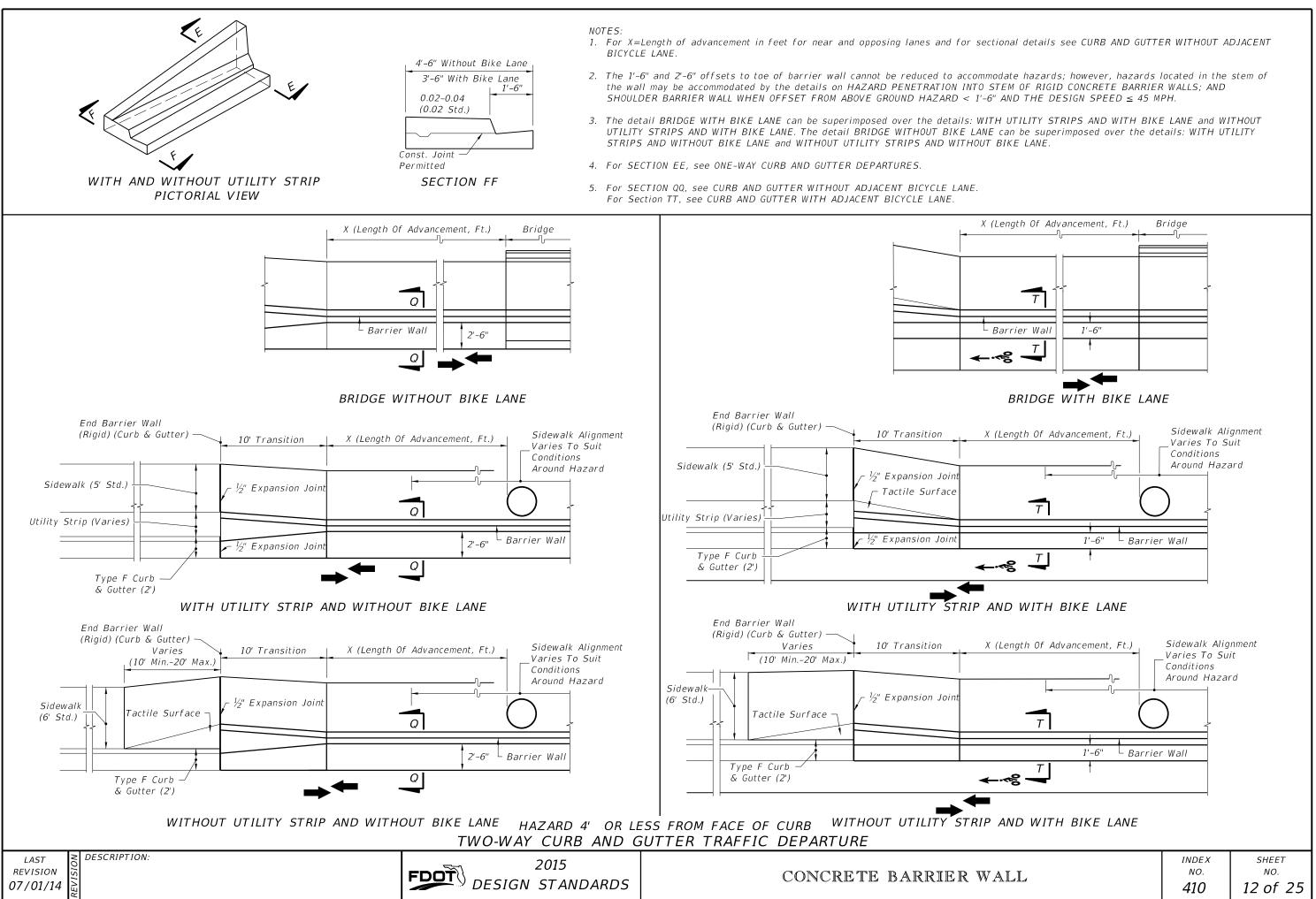
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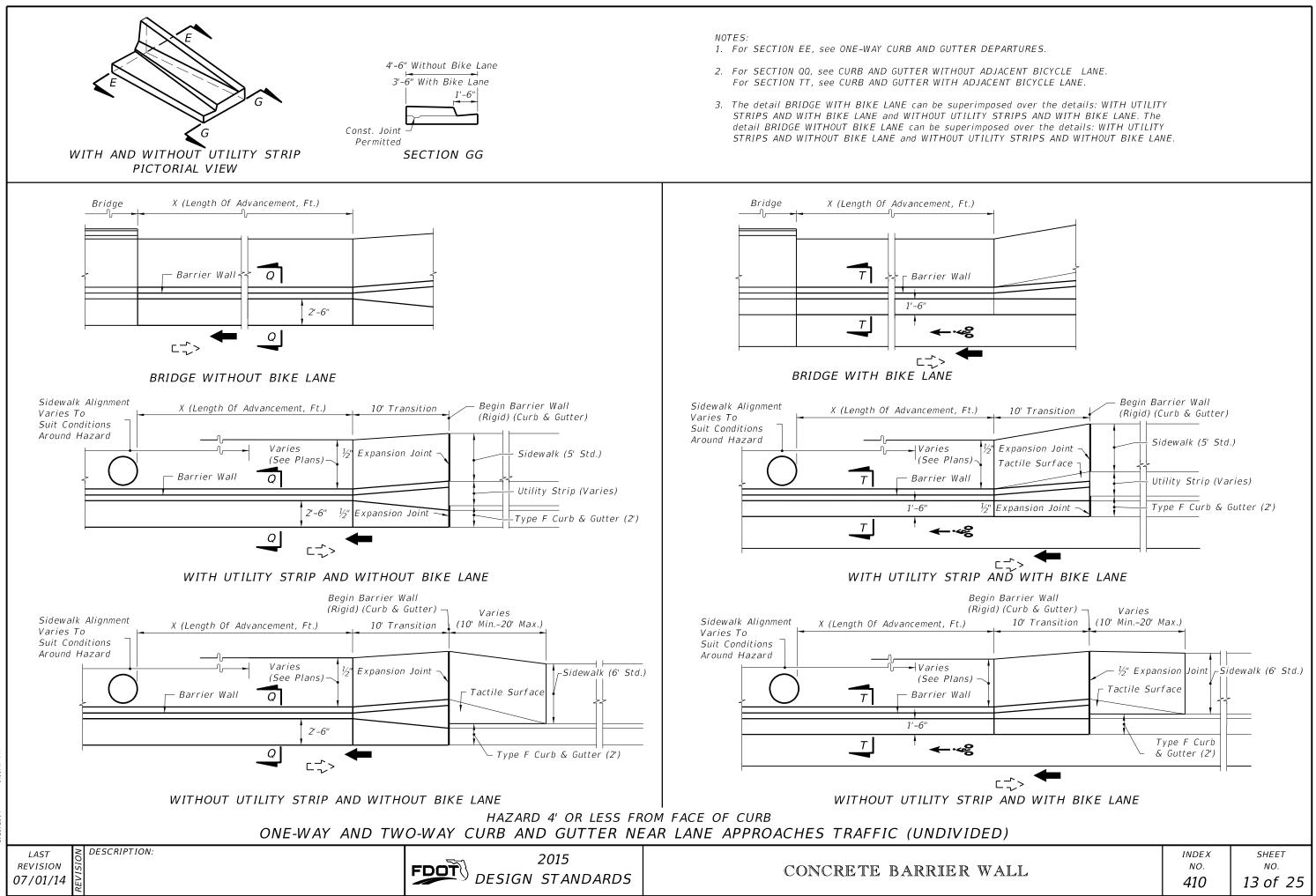




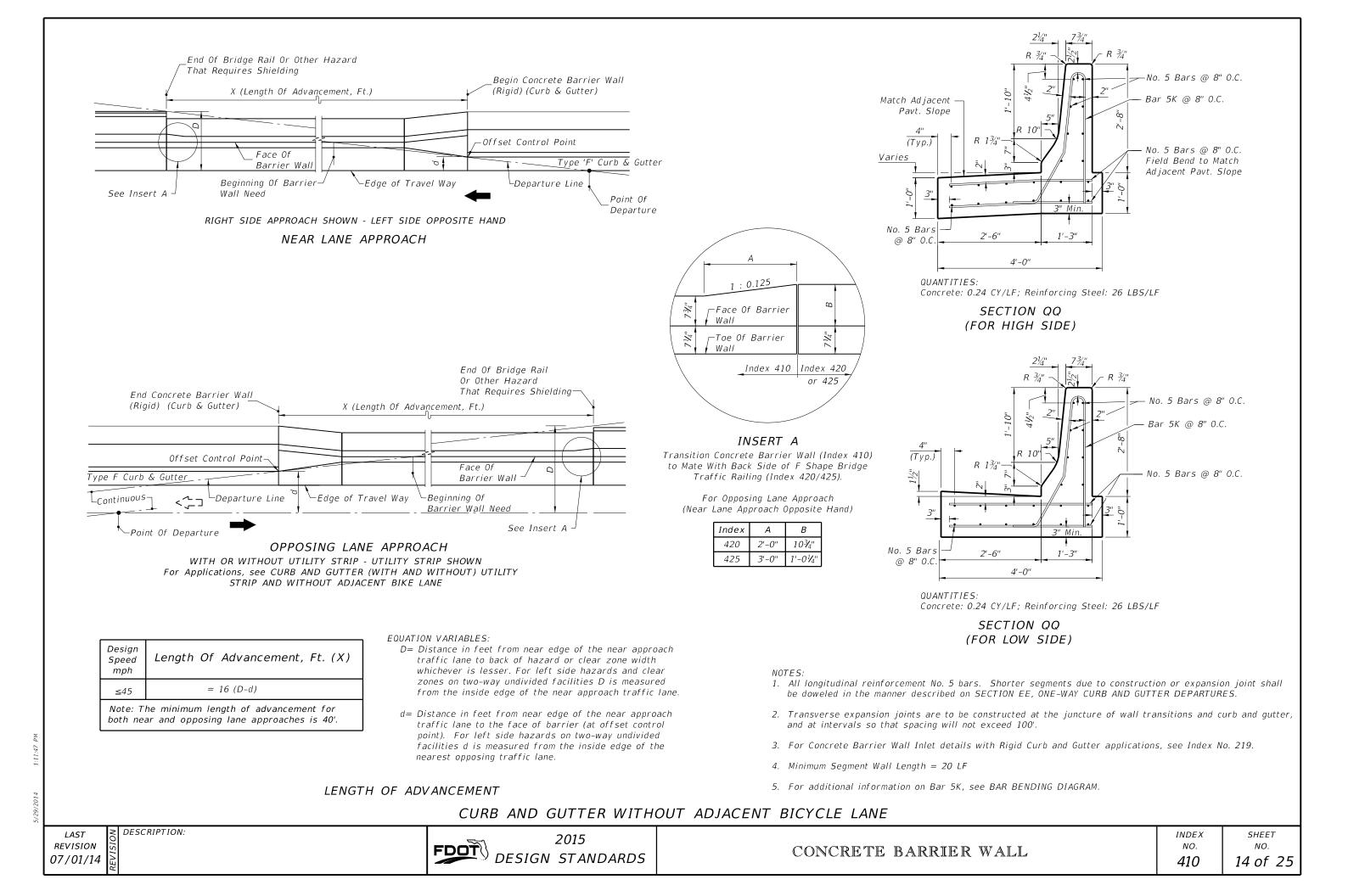


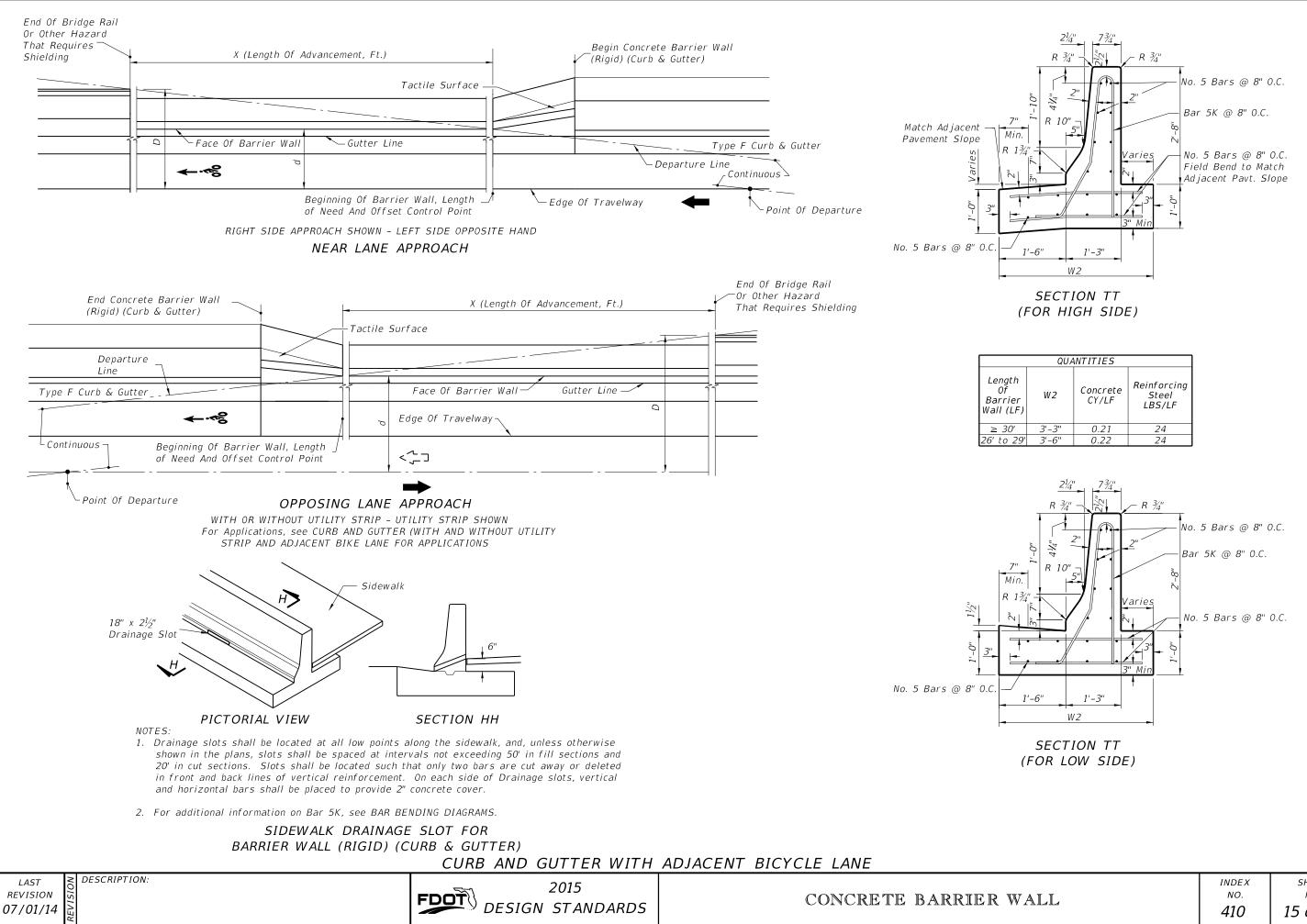


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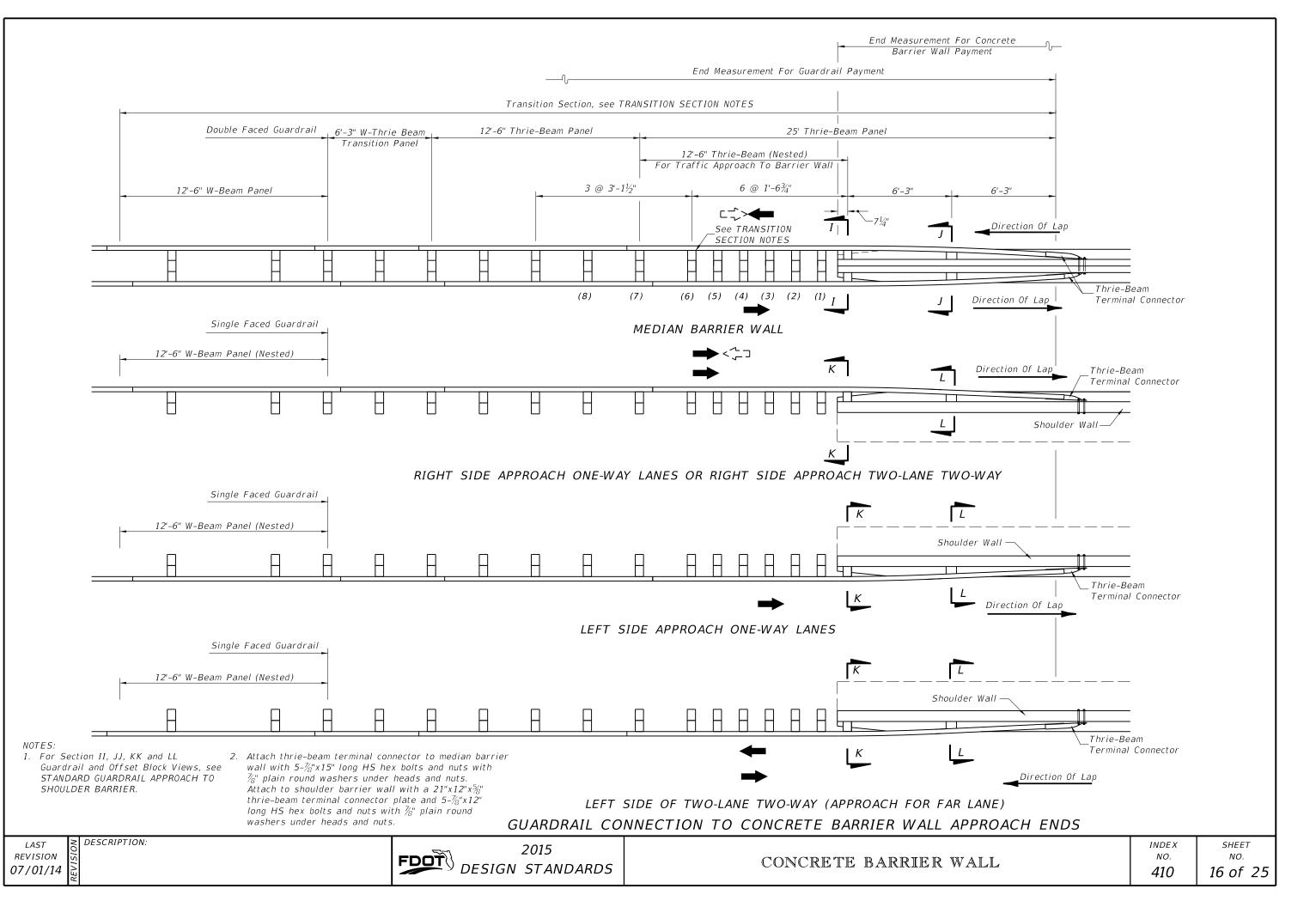
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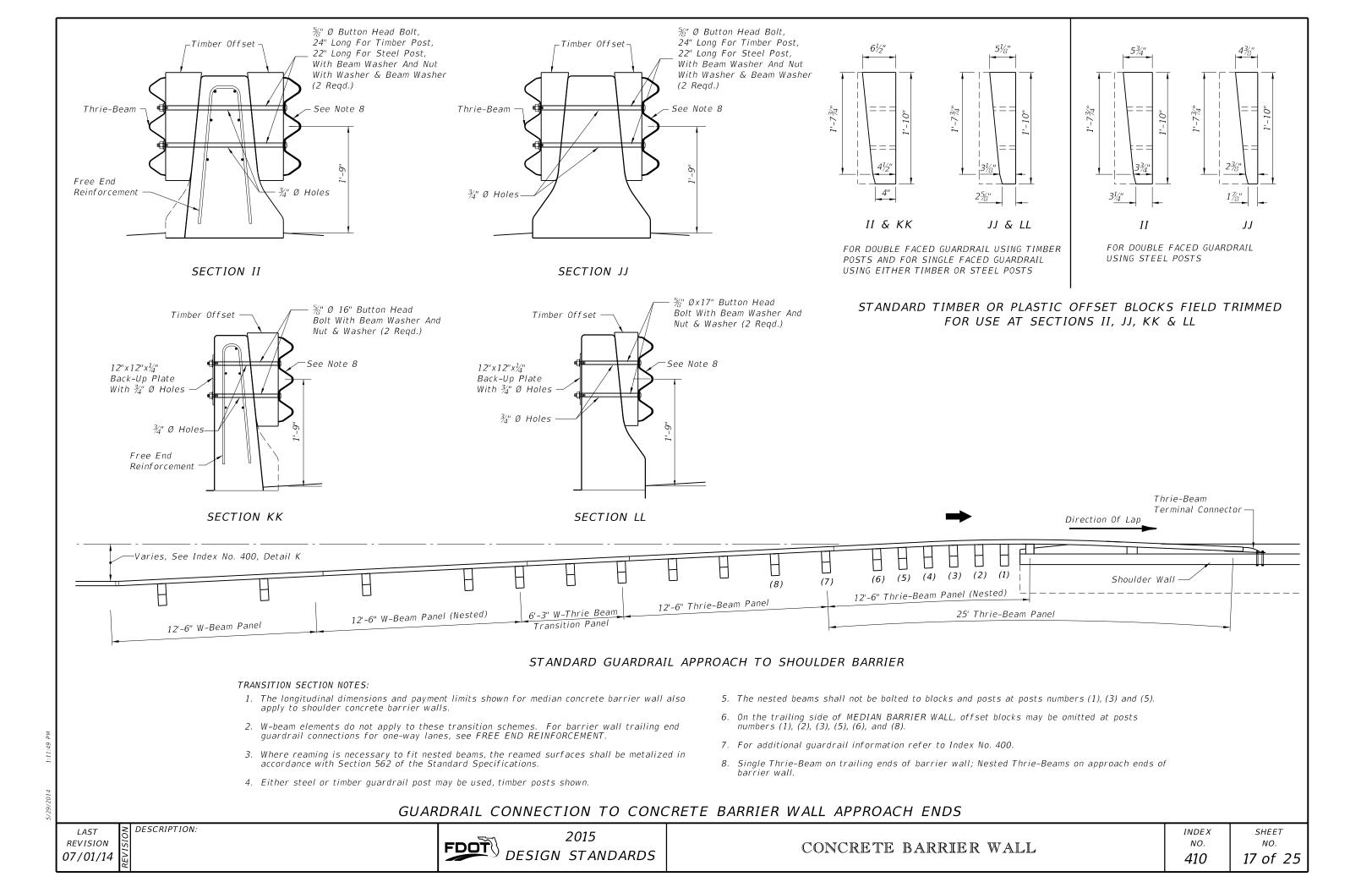


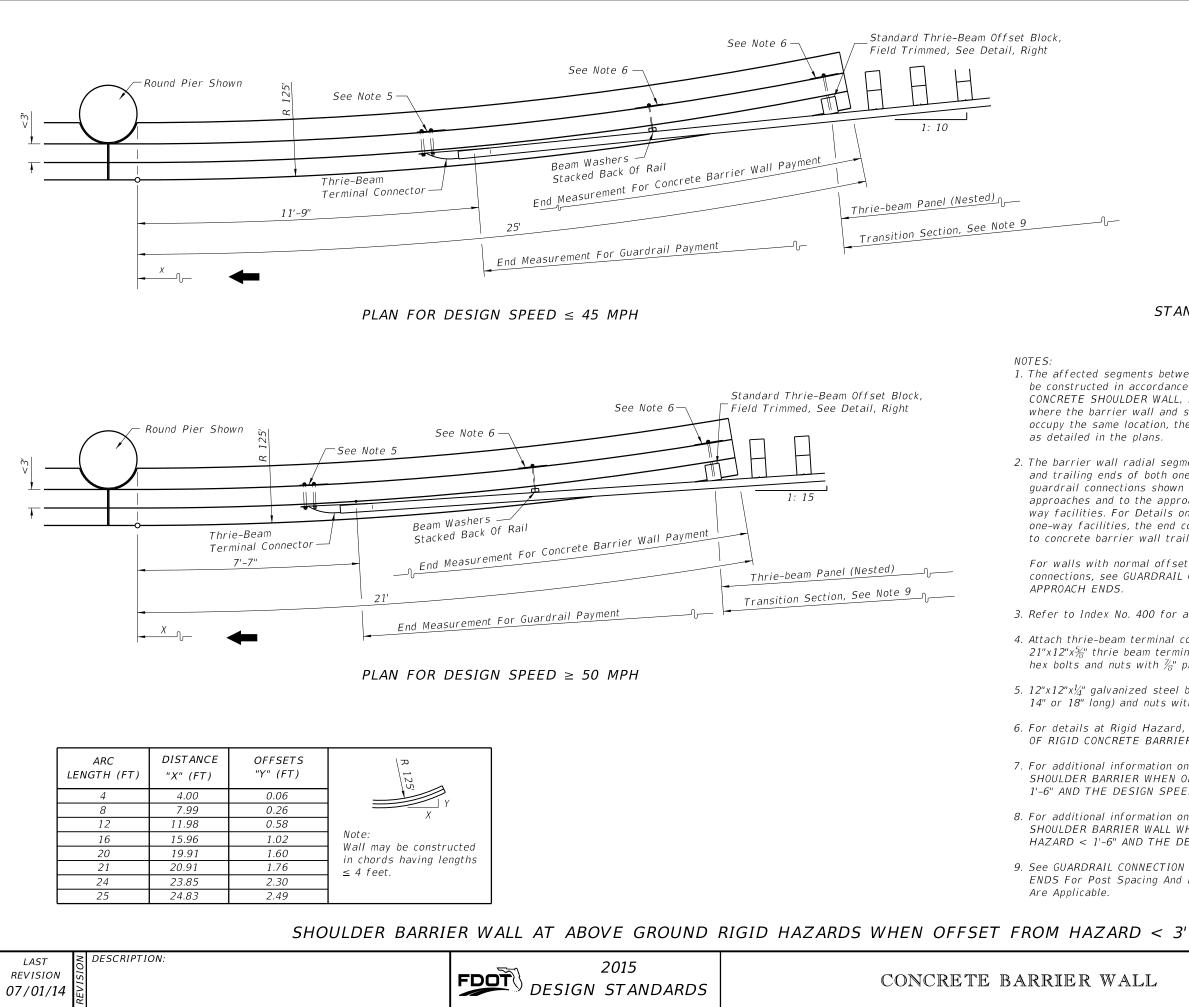
QUANTITIES		
W2	Concrete CY/LF	Reinforcing Steel LBS/LF
3'-3"	0.21	24
3'-6"	0.22	24

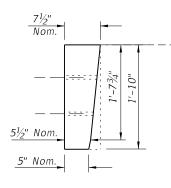
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5/29/2014







FOR USE WITH EITHER 1: 10 OR 1: 15 GUARDRAIL TRANSITIONS

# STANDARD THRIE-BEAM OFFSET BLOCK (FIELD TRIMMED)

1. The affected segments between bent supports or pier columns shall be constructed in accordance with the detail for REINFORCED CONCRETE SHOULDER WALL, Section QQ, or Section TT. In cases where the barrier wall and slope pavement or other structure would occupy the same location, the wall and structure are to be modified

2. The barrier wall radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane twoway facilities. For Details on trailing ends of two-way multilane and one-way facilities, the end connection on W-Beam guardrail connection to concrete barrier wall trailing ends may be used.

For walls with normal offsets from hazards and their guardrail connections, see GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL

3. Refer to Index No. 400 for additional guardrail information.

4. Attach thrie-beam terminal connector to shoulder barrier wall with a 21"x12"x%" thrie beam terminal connector plate and 5-%"x12" long HS hex bolts and nuts with  $\frac{7}{8}$ " plain round washers under heads and nuts.

5.  $12" \times 12" \times \frac{1}{4}"$  galvanized steel back-up plate with  $\frac{5}{8}"$  post bolts (either 14" or 18" long) and nuts with %" plain round washers under nuts.

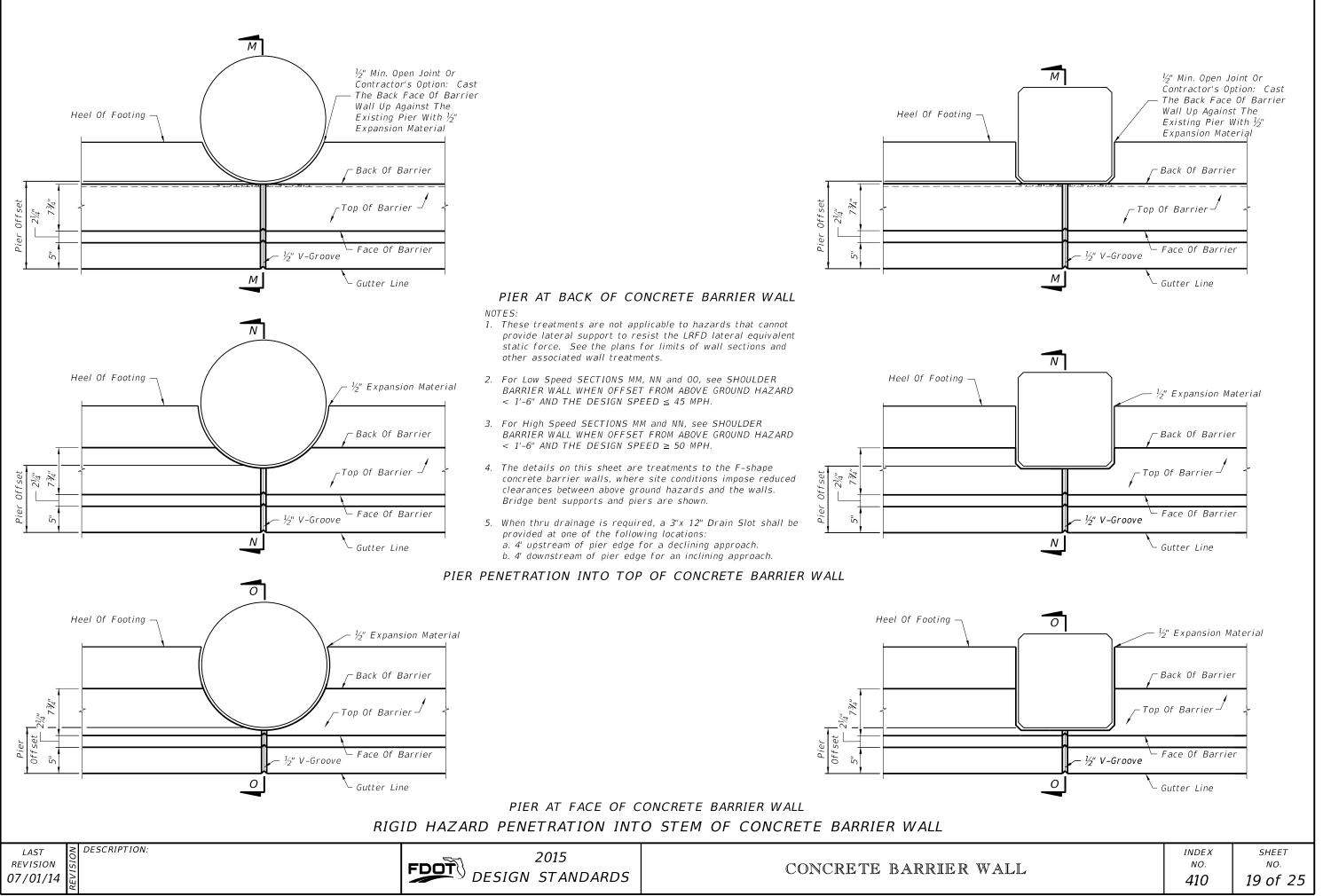
6. For details at Rigid Hazard, see HAZARD PENETRATION INTO STEM OF RIGID CONCRETE BARRIER WALLS.

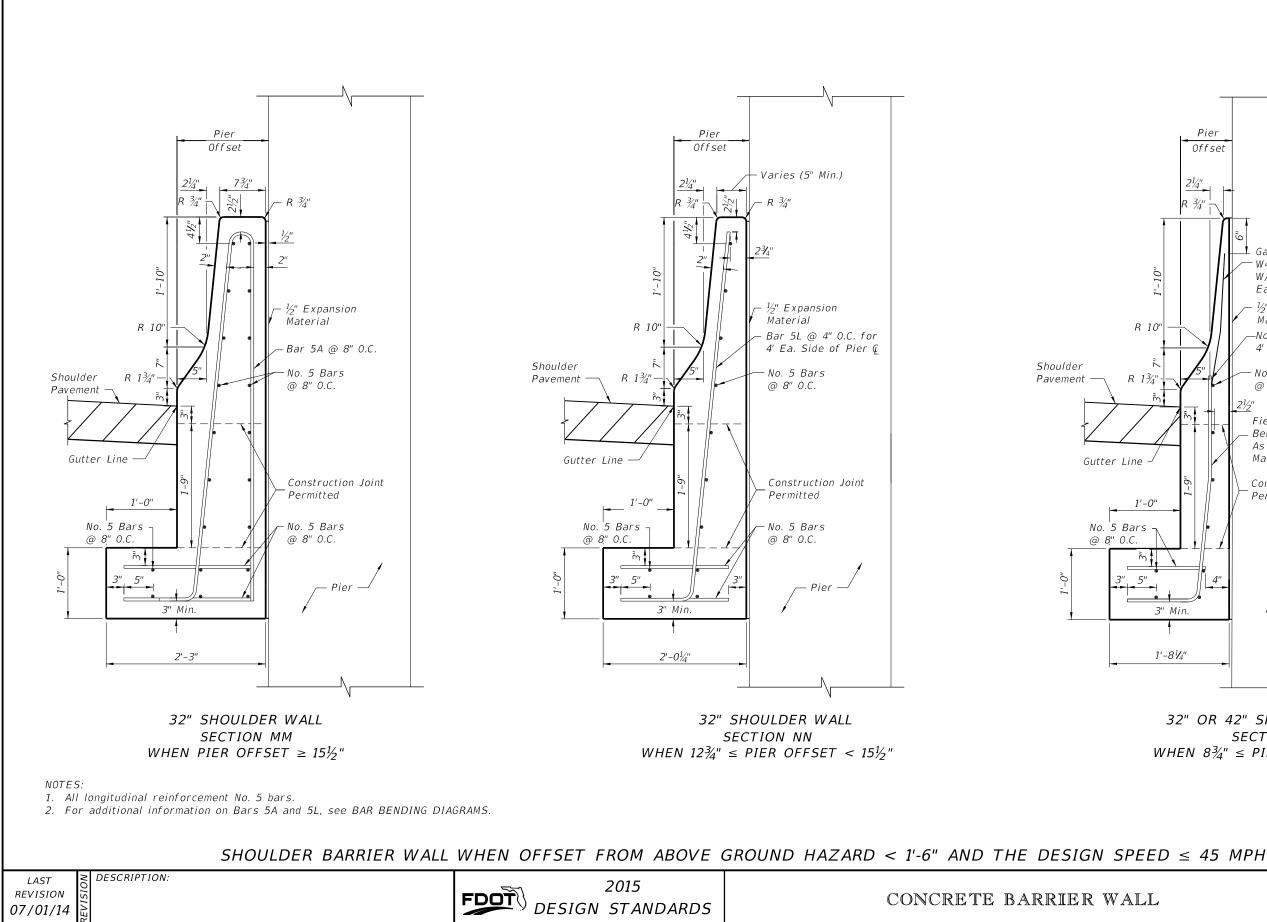
7. For additional information on PLAN FOR DESIGN SPEED ≤ 45 MPH, see SHOULDER BARRIER WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6" AND THE DESIGN SPEED  $\leq$  45 MPH.

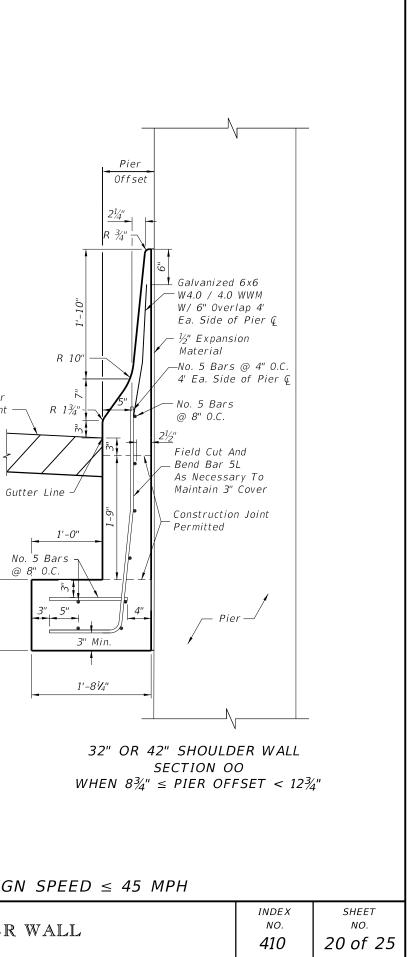
8. For additional information on PLAN FOR DESIGN SPEED  $\geq$  50 MPH, see SHOULDER BARRIER WALL WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6" AND THE DESIGN SPEED  $\geq$  50 MPH.

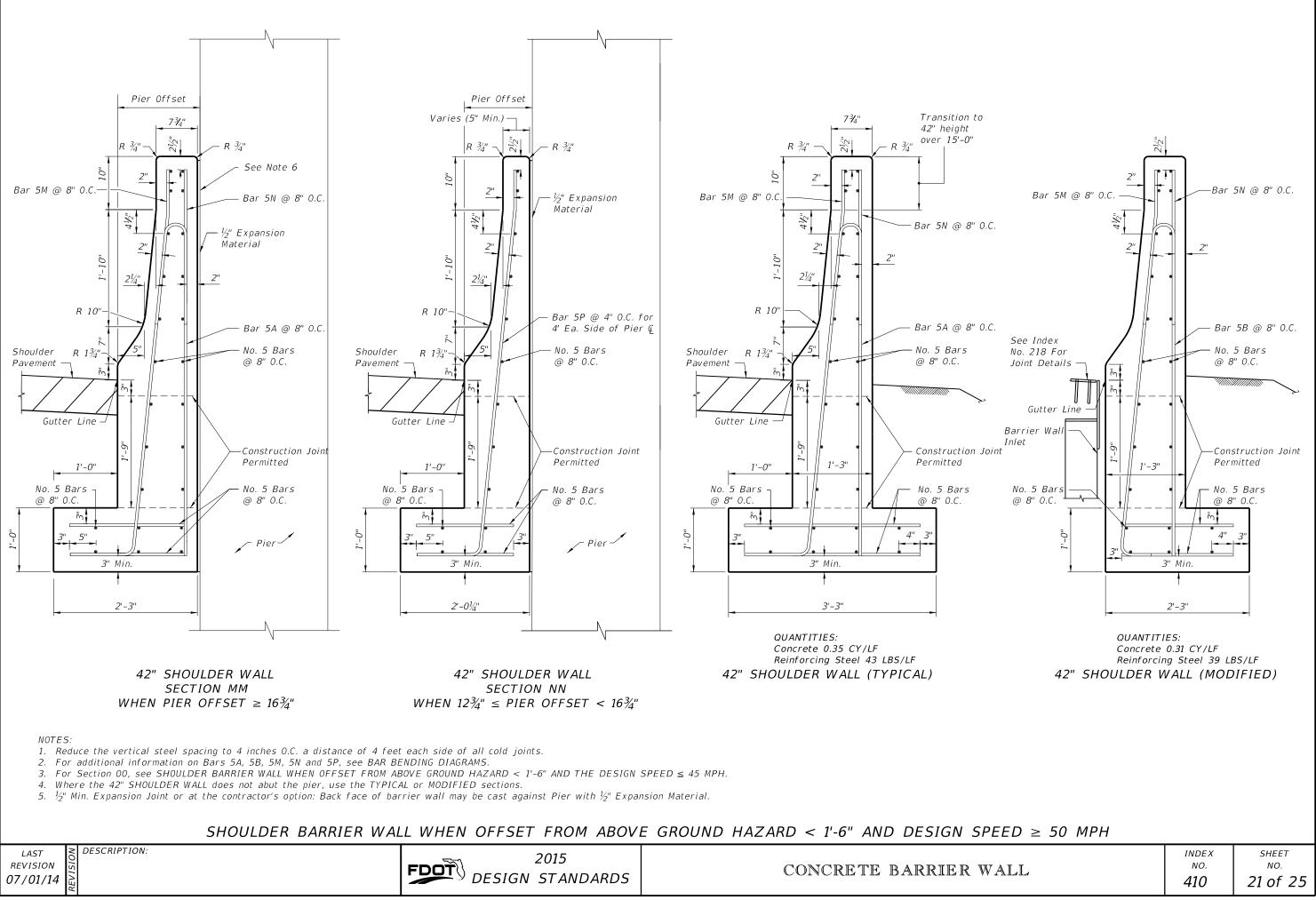
9. See GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL APPROACH ENDS For Post Spacing And Bolt Connections, Steel Or Timber Posts

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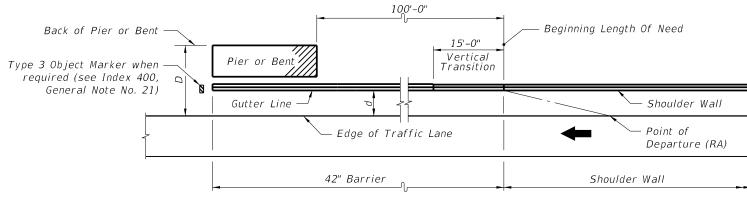


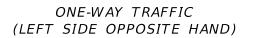


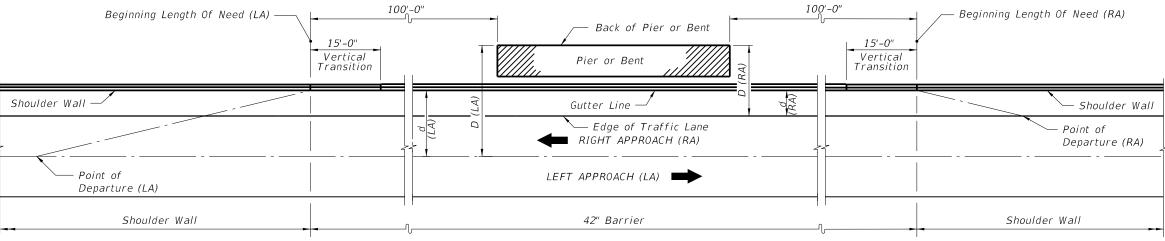


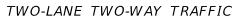


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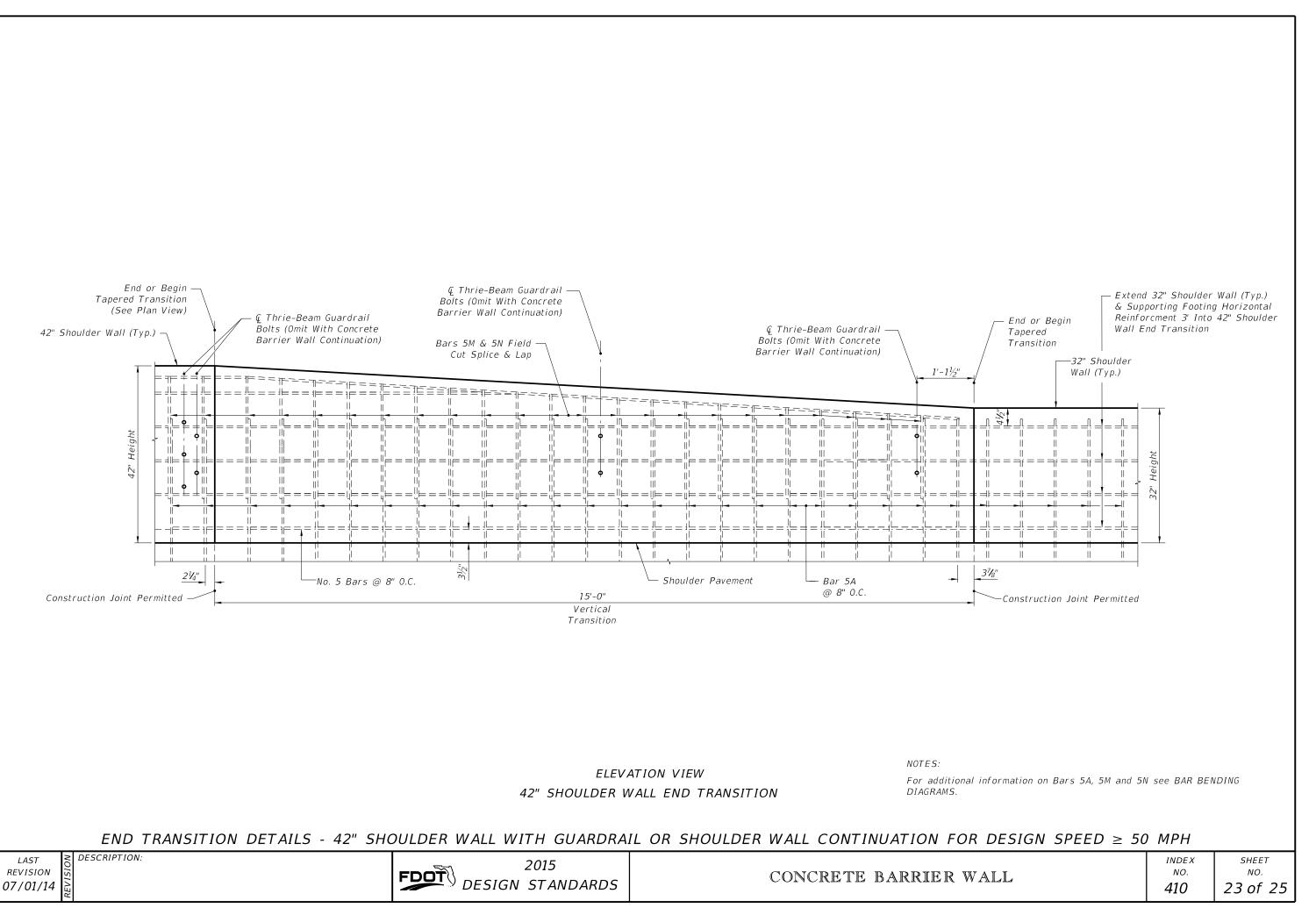


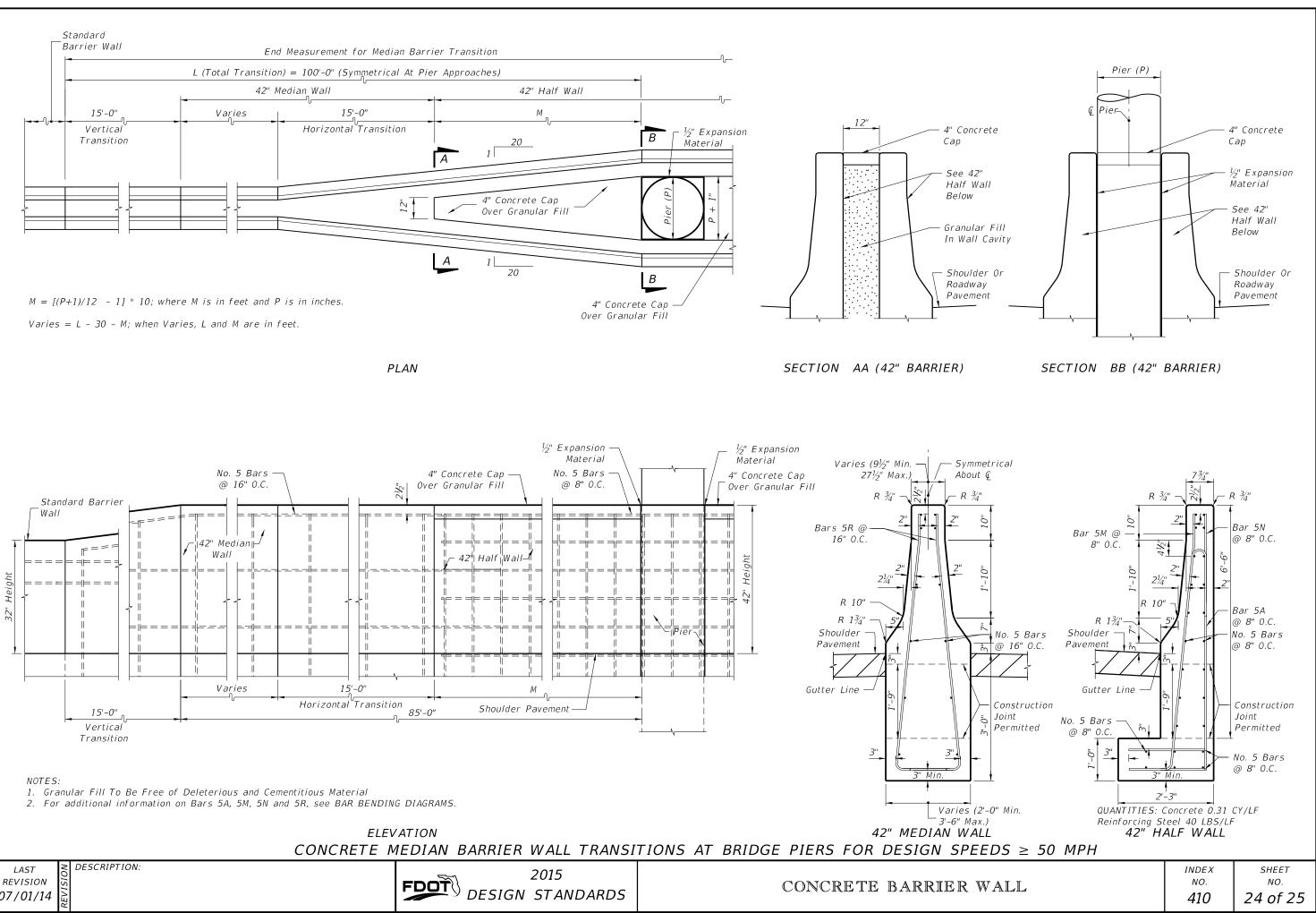
SHOULDER BARRIER WALL WHEN OFFSET FROM ABOVE GROUND HAZARD < 1'-6" AND DESIGN SPE

SION	DESCRIPTION:	2015	CONCRETE BARRIER WA
REVI		DESIGN STANDARDS	

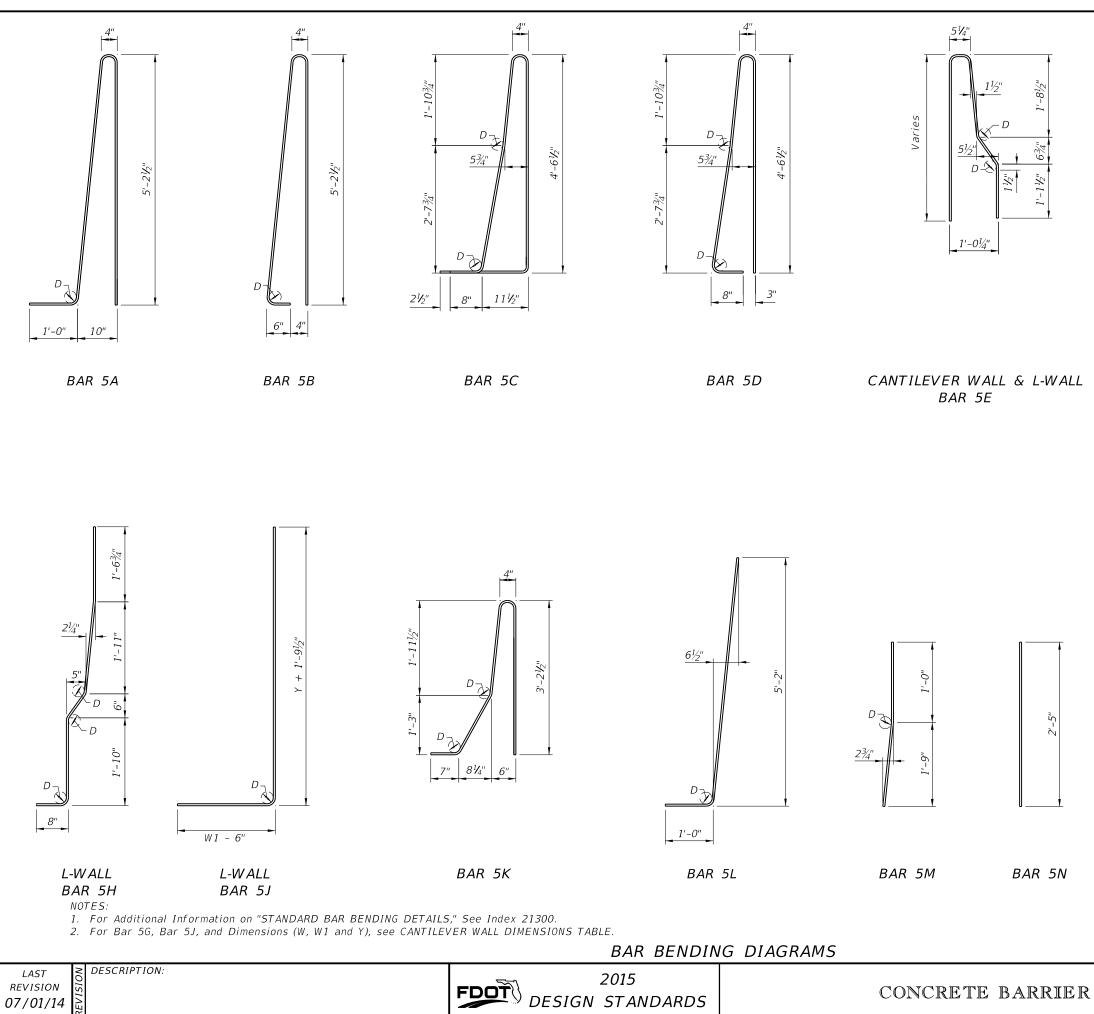
LAST REVISION 07/01/14

$ED \ge 50 MPH$		
ALL	index no. <b>410</b>	<sup>SHEET</sup> NO. <b>22 of 25</b>

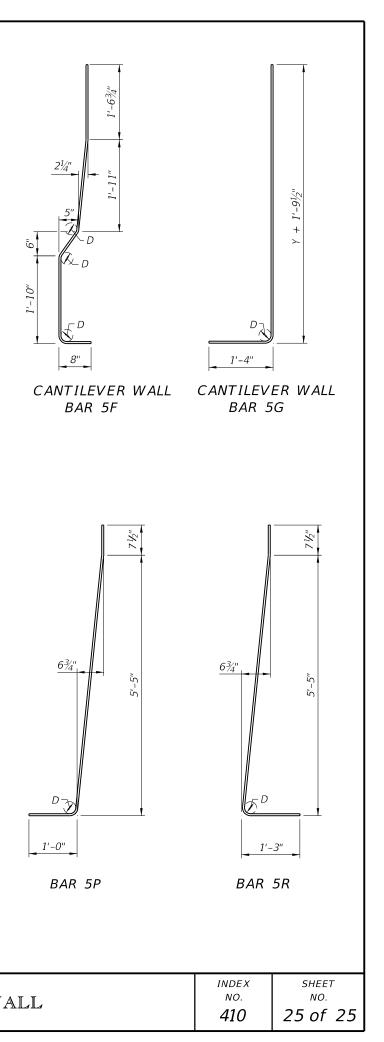




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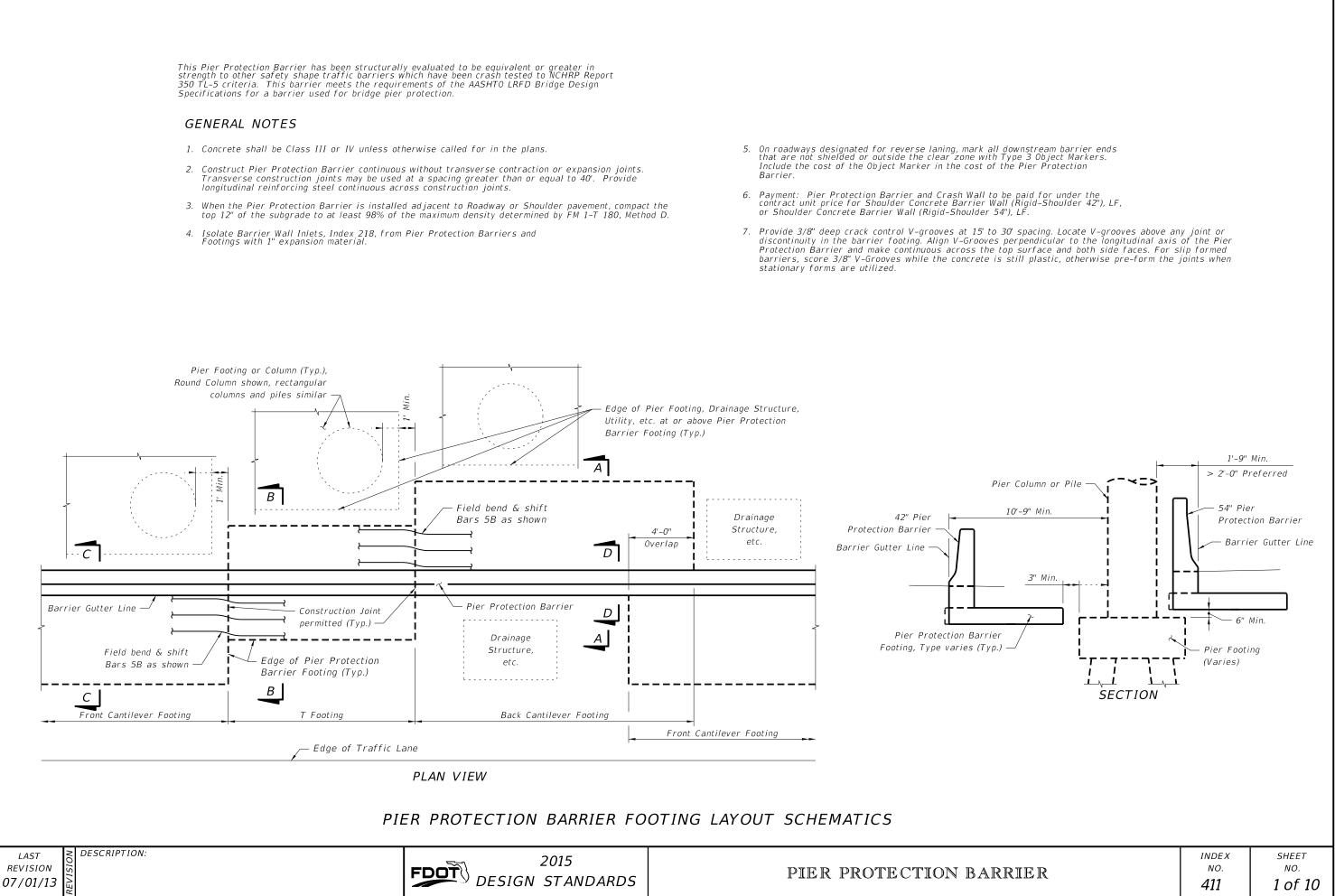


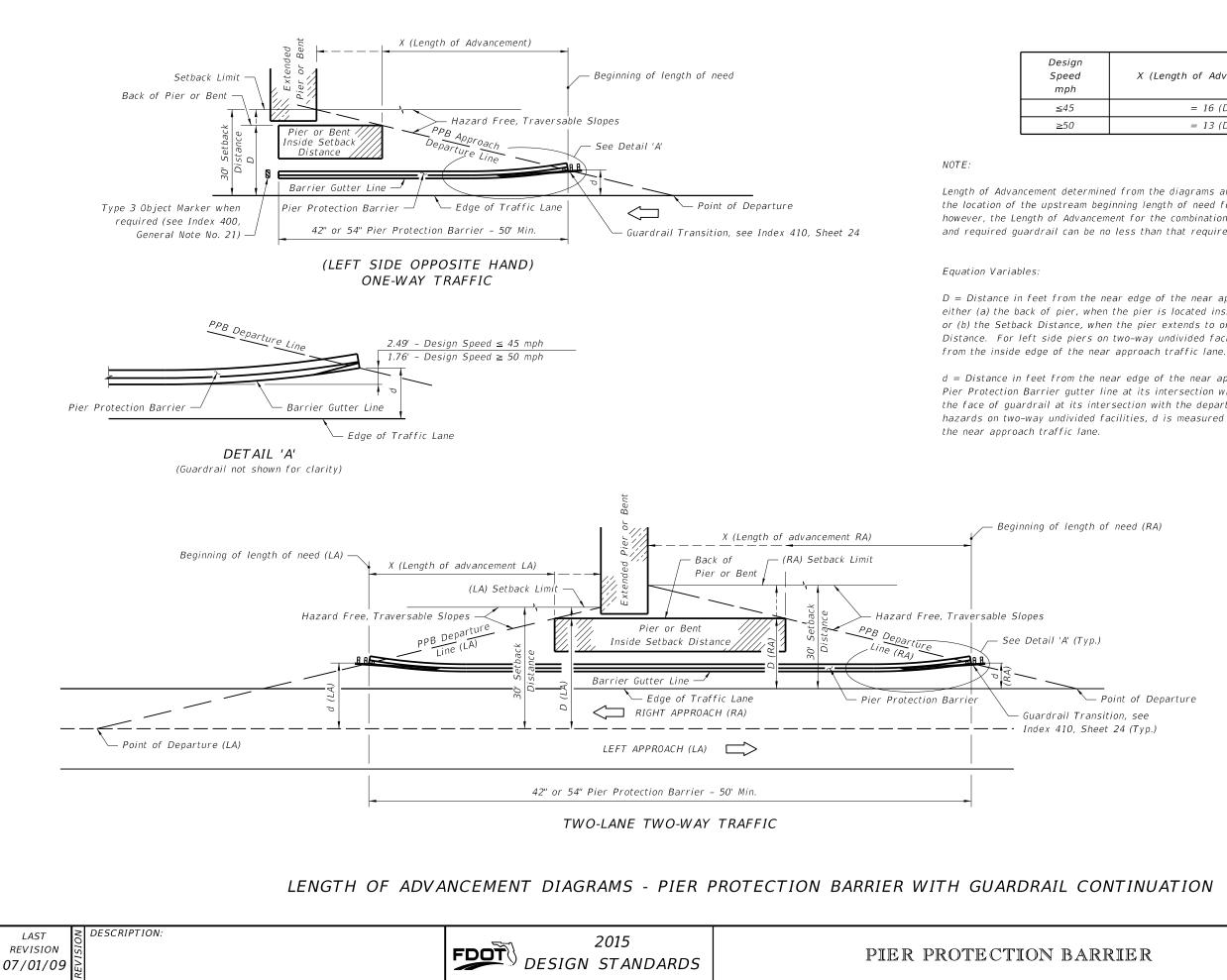
CONCRETE BARRIER WALL



350 TL-5 criteria. This barrier meets the requirements of the AASHTO LRFD Bridge Design Specifications for a barrier used for bridge pier protection.

- Barrier
- stationary forms are utilized.





X (Length of Advancement) H	₹t.
= 16 (D-d)	
= 13 (D-d)	

Length of Advancement determined from the diagrams and equations shown establishes the location of the upstream beginning length of need for a Pier Protection Barrier, however, the Length of Advancement for the combination of Pier Protection Barrier and required guardrail can be no less than that required by other details of Index 400.

D = Distance in feet from the near edge of the near approach traffic lane to either (a) the back of pier, when the pier is located inside the Setback Distance or (b) the Setback Distance, when the pier extends to or goes beyond the Setback Distance. For left side piers on two-way undivided facilities, D is measured

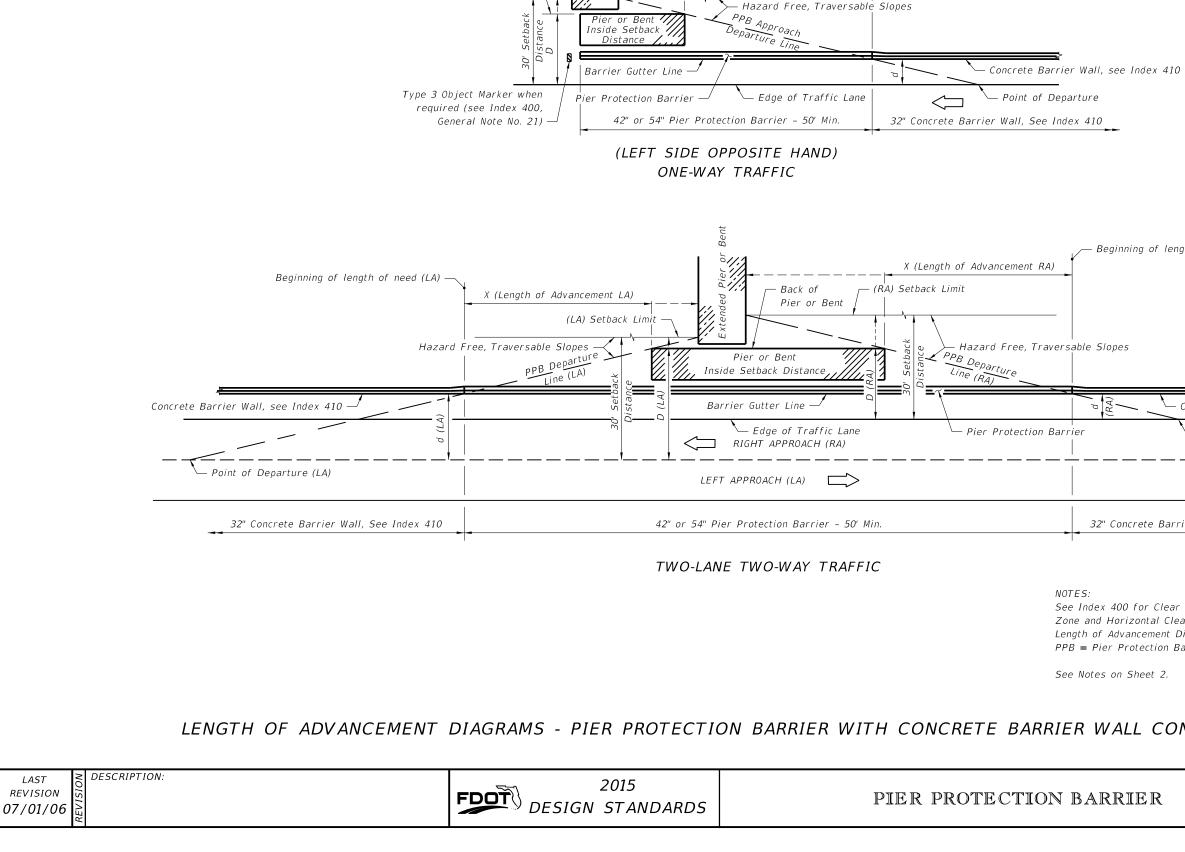
d = Distance in feet from the near edge of the near approach traffic lane to the Pier Protection Barrier gutter line at its intersection with the departure line or the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of

— Point of Departure

NOTE: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams.

*PPB* = *Pier Protection Barrier* 

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Setback Limit

Back of Pier or Bent

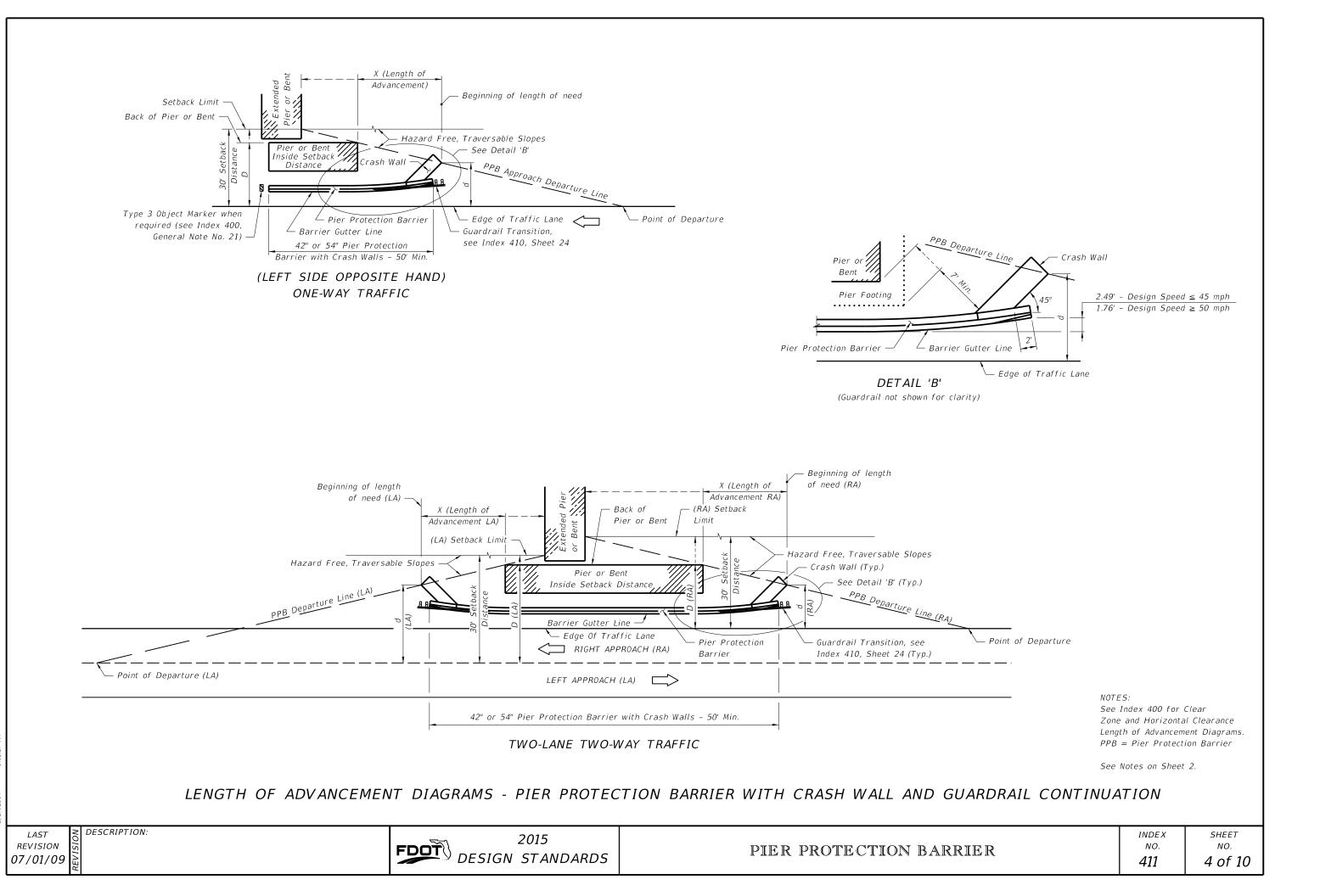
X (Length of Advancement)

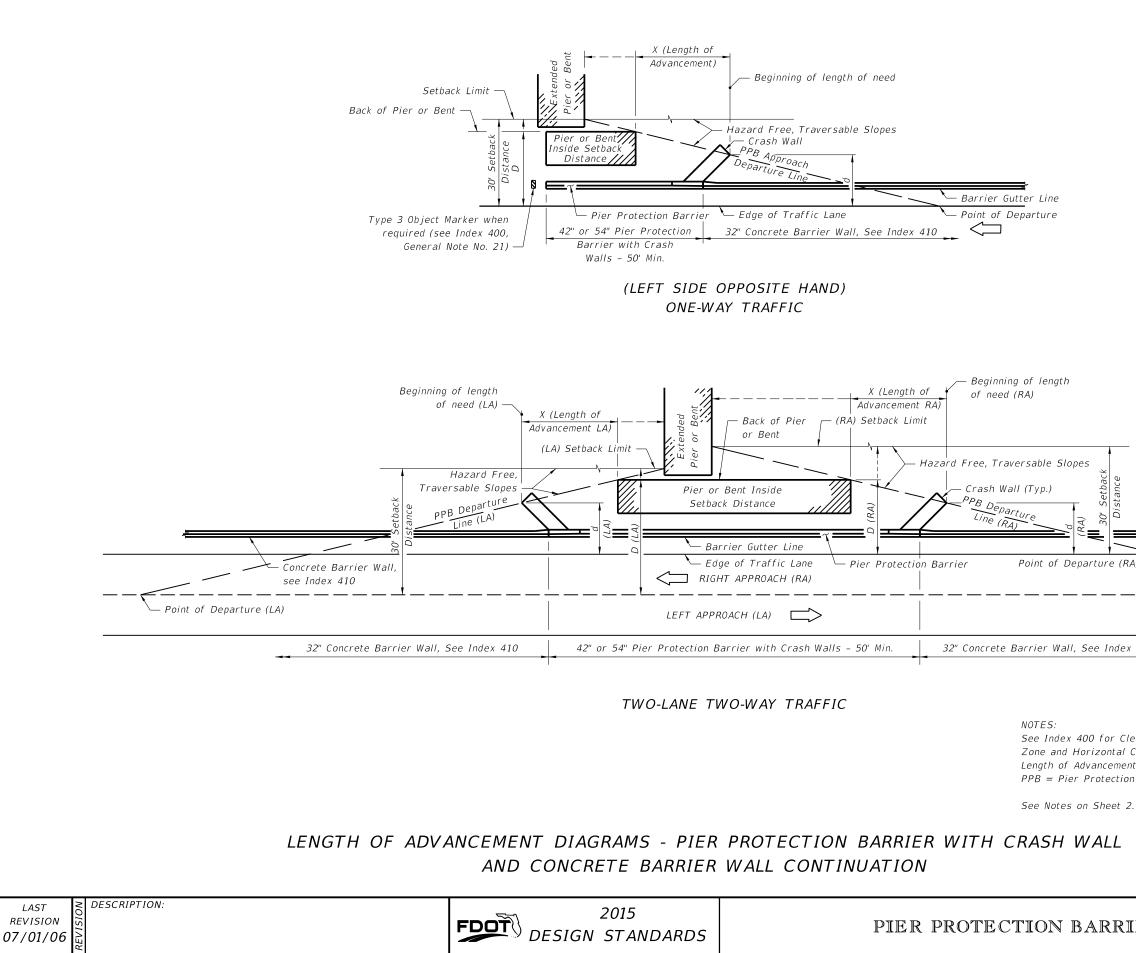
Beginning of length of need

- Beginning of length of need (RA)

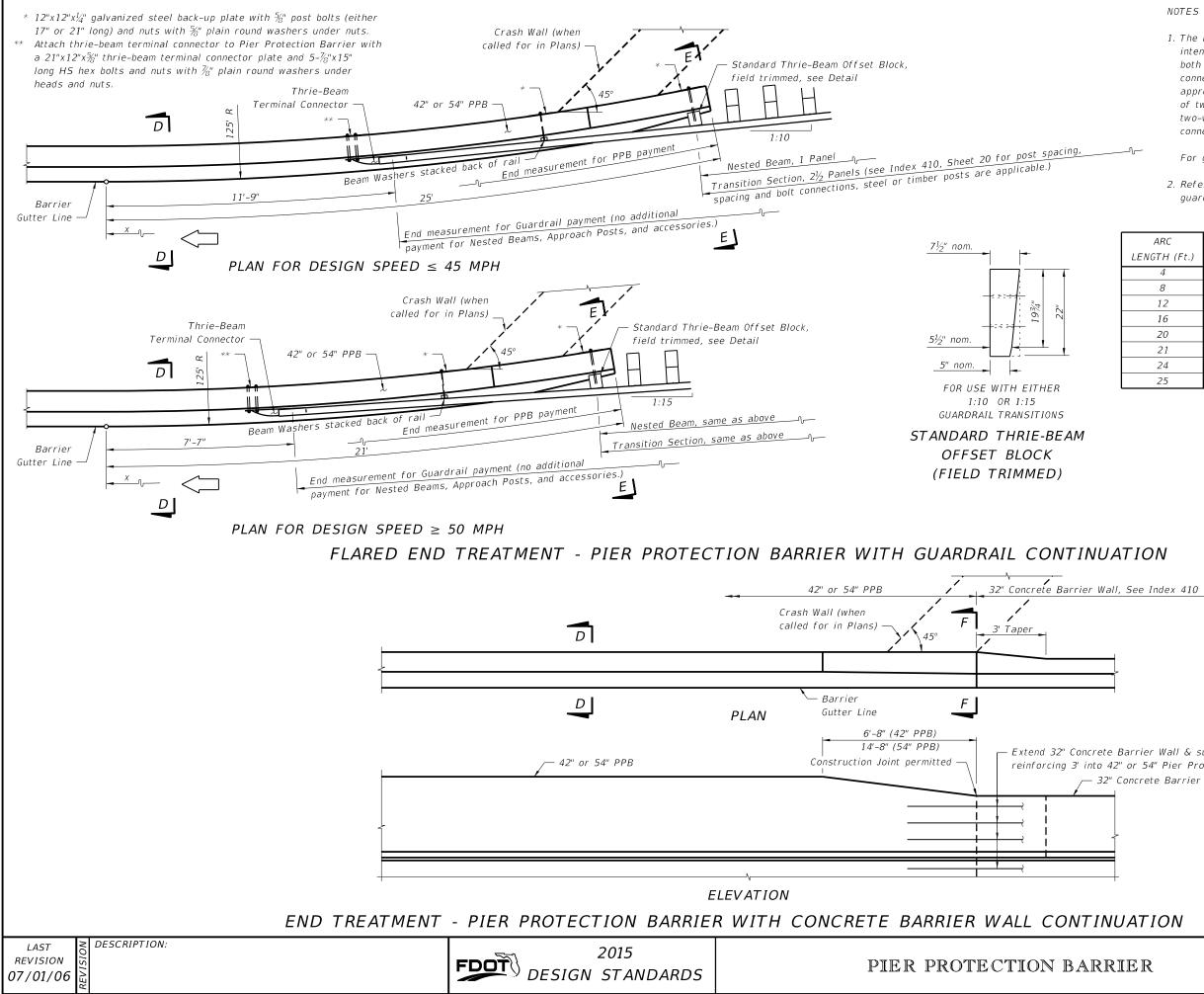
Concrete Barrier Wall, se	ee Index 410	
Point of Departure		
ncrete Barrier Wall, See Index 410	-	
0 for Clear Tizontal Clearance Vancement Diagrams. Protection Barrier		
Sheet 2.		
LL CONTINUATION		
		SHE

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_		
RA) — Concrete Barrier Wall See Index 410 — — — —	,	
ex 410		
Clear I Clearance ent Diagrams. ion Barrier		
2.		
IER	index no. <b>411</b>	<sup>sнеет</sup> <sup>NO.</sup> <b>5 of 10</b>



# NOTES

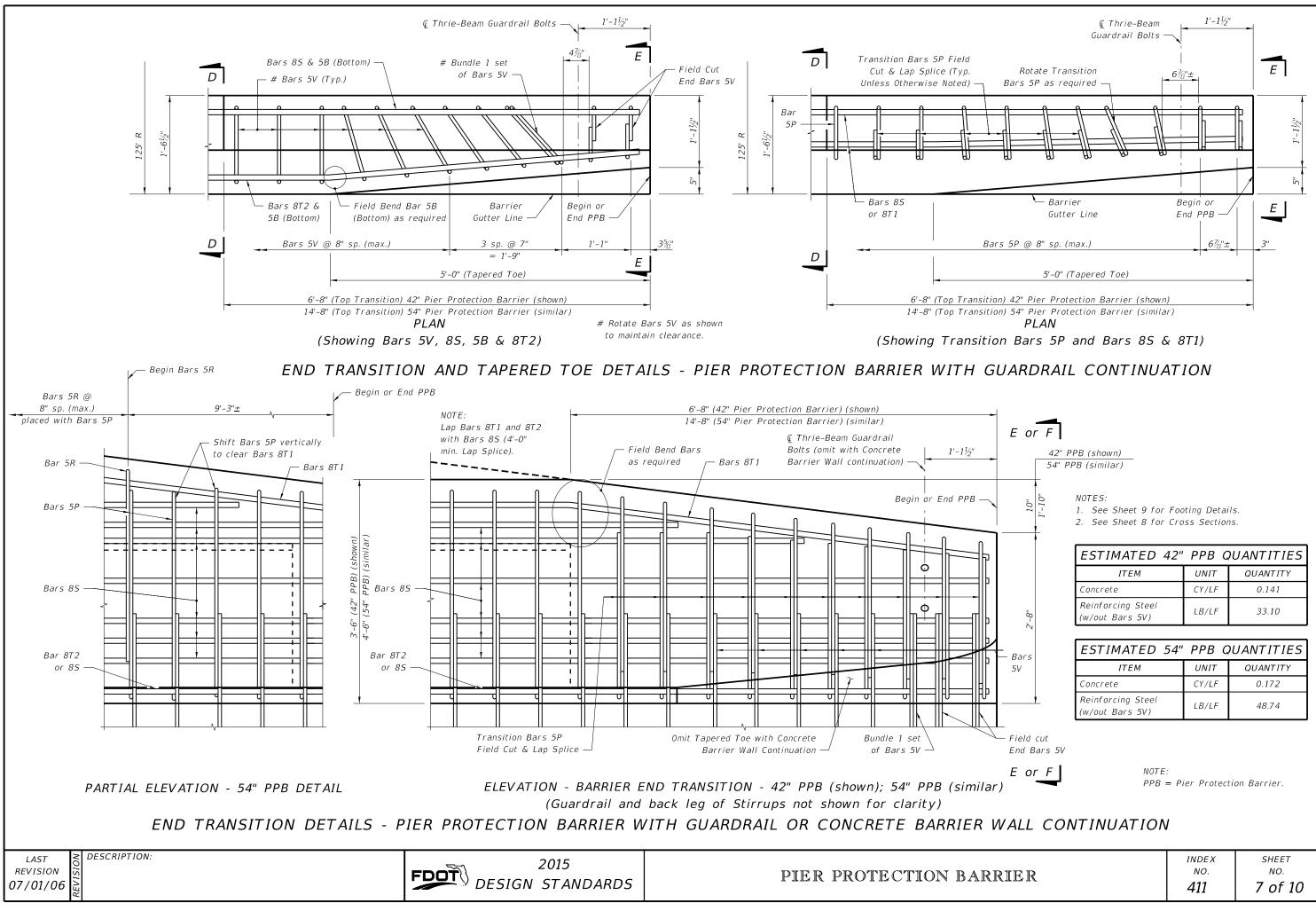
- 1. The Pier Protection Barrier radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane two-way facilities. On trailing ends of two-way multilane and one-way facilities the end connection on Index 410, Sheet 2 may be used.
- For guardrail connections, see Index 410, Sheet 20.
- 2. Refer to Index No. 400 Detail J for additional guardrail information.

ARC	DISTANCE	OFFSETS "y"	
LENGTH (Ft.)	"x" (Ft.)	"y" (Ft.)	125
4	4.00	0.06	
8	7.99	0.26	Y
12	11.98	0.58	X
16	15.96	1.02	Note:
20	19.91	1.60	Barrier may be constructed in chords having lengths ≤4 feet.
21	20.91	1.76	
24	23.85	2.30	
25	24.83	2.49	

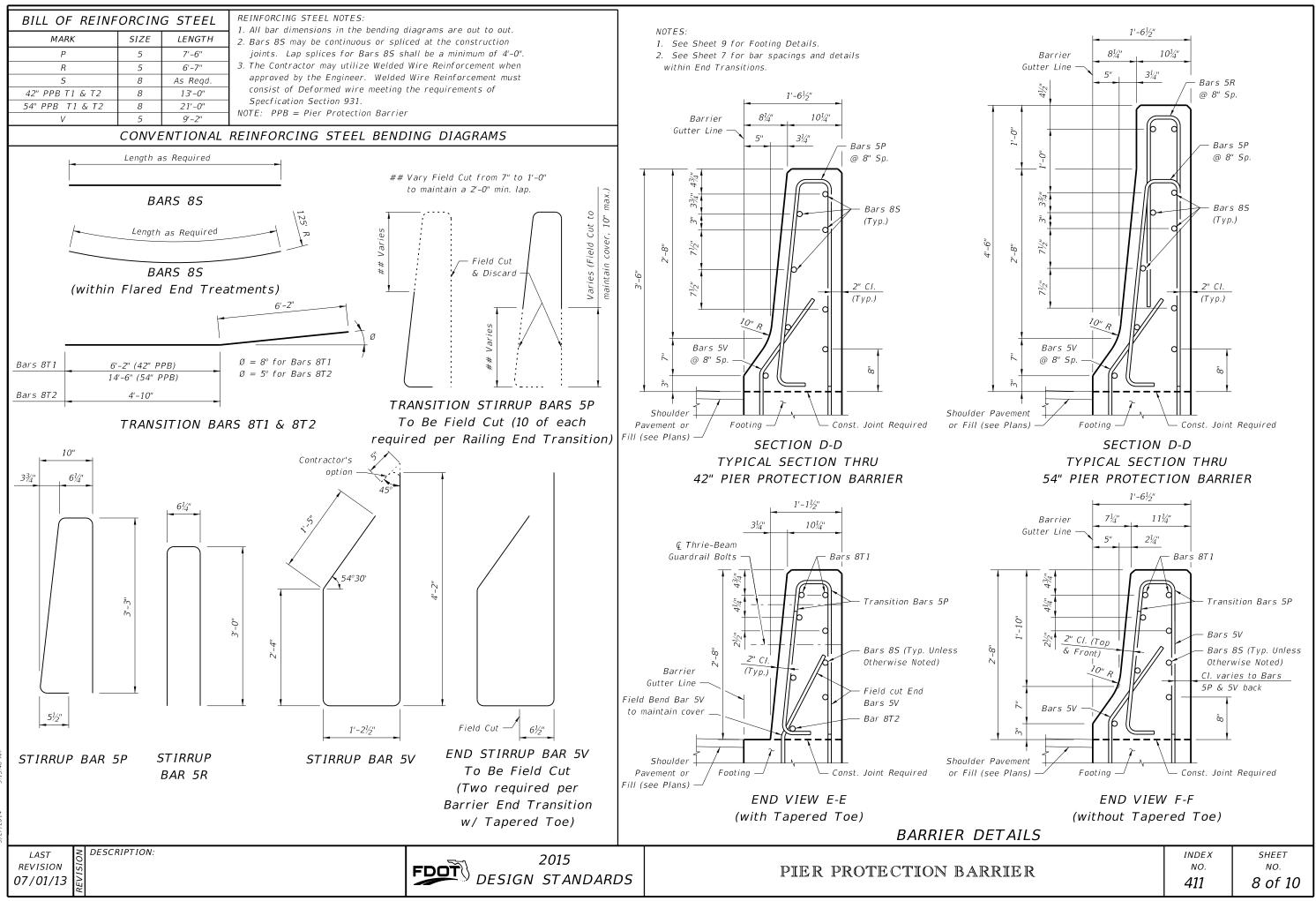
Extend 32" Concrete Barrier Wall & supporting footing horizontal reinforcing 3' into 42" or 54" Pier Protection Barrier (Typ.) - 32" Concrete Barrier Wall, See Index 410

> NOTE: PPB = Pier Protection Barrier.

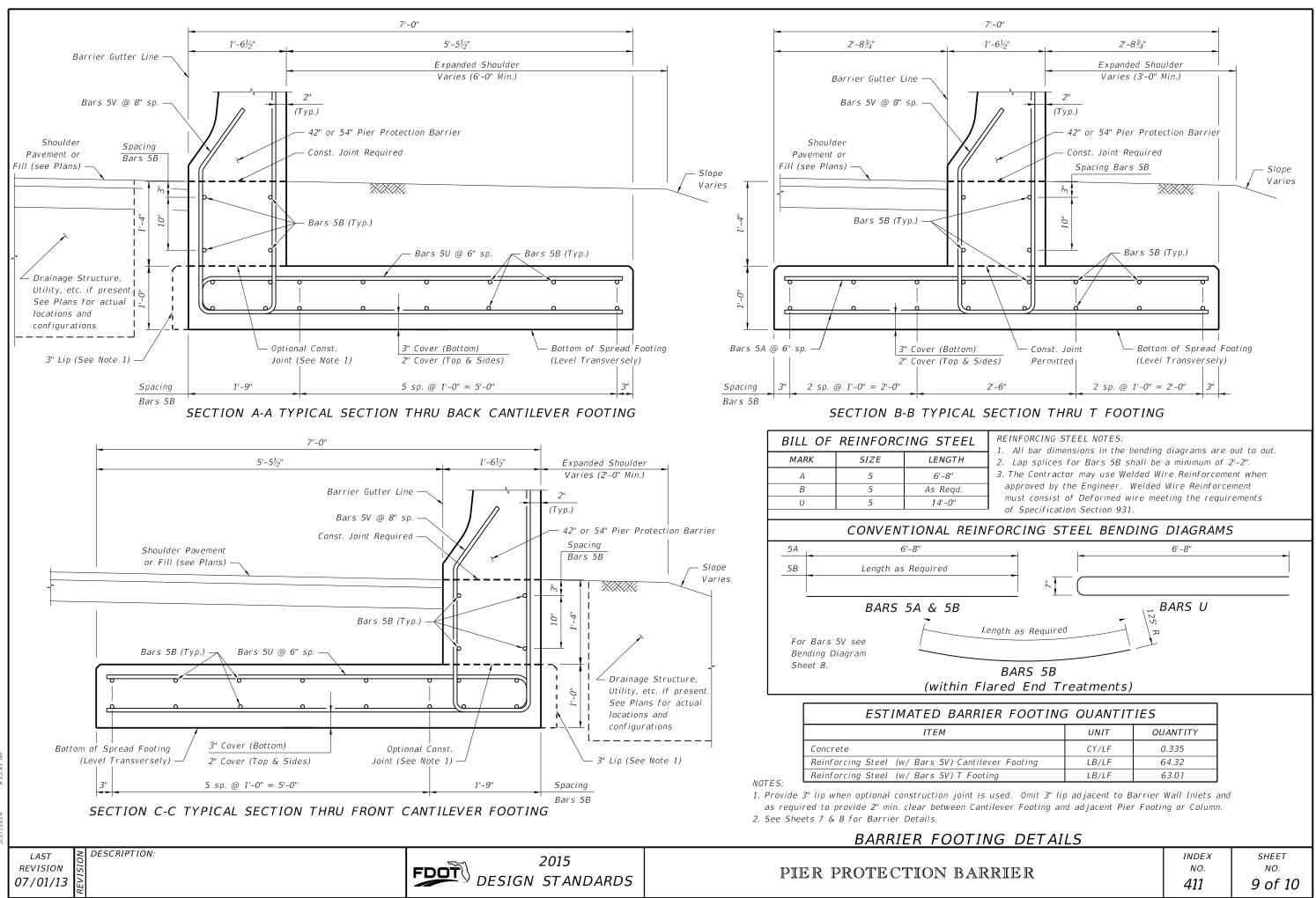
	INDEX	SHEET
RIER	NO.	NO.
	411	6 of 10

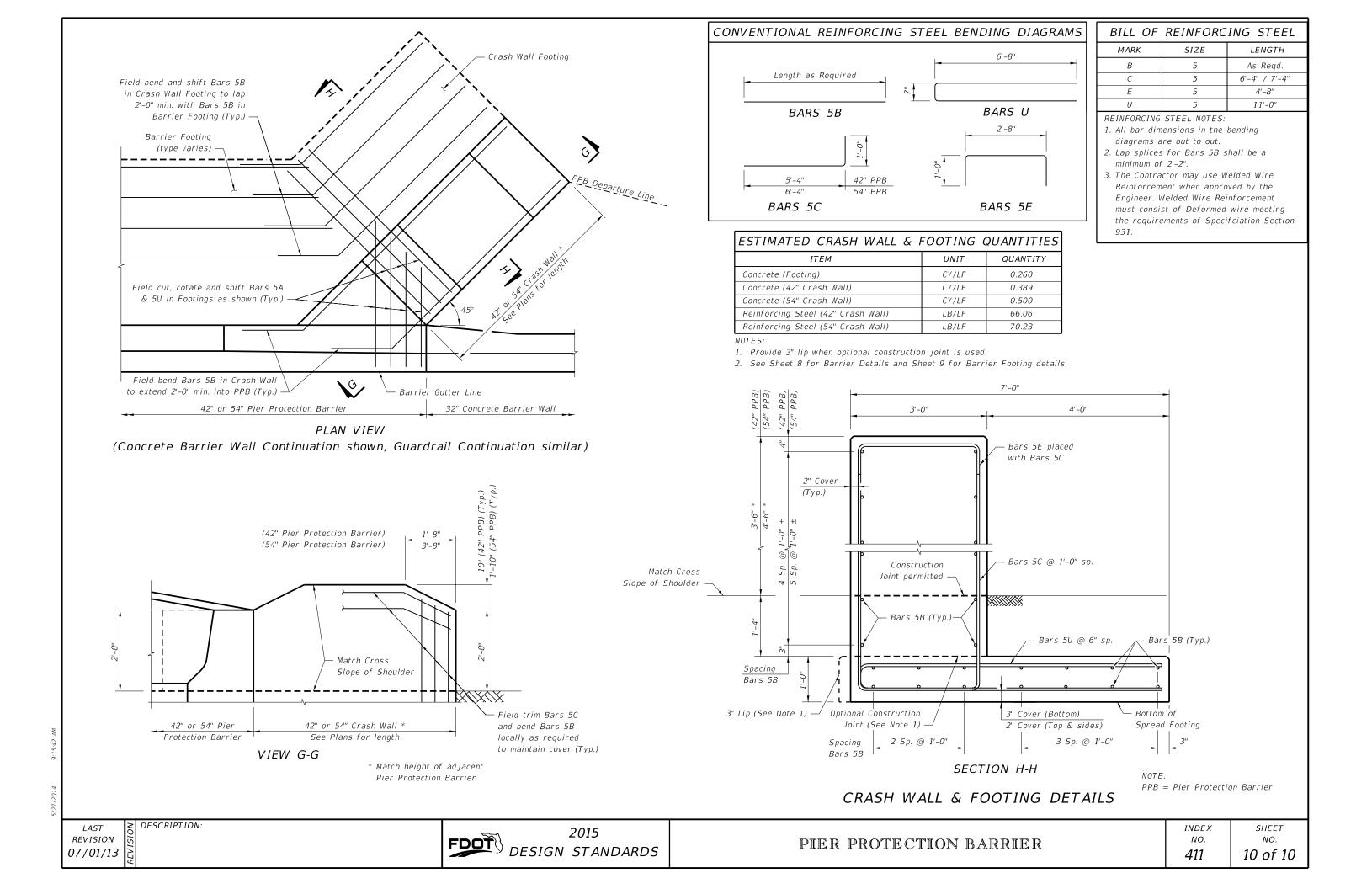


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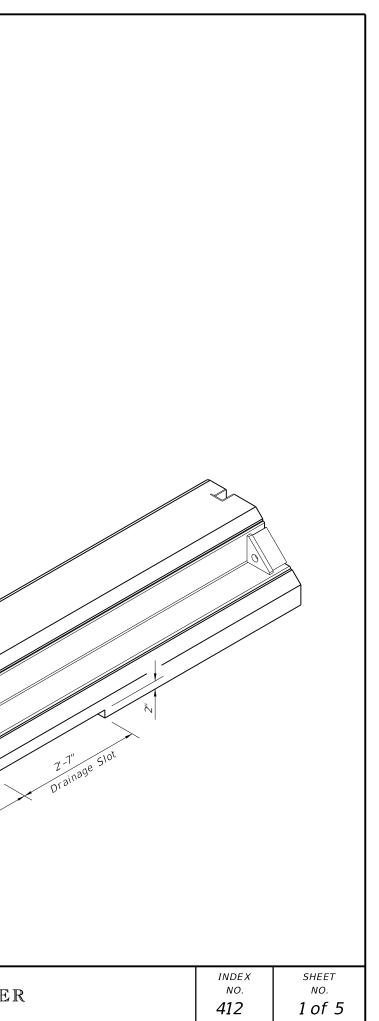


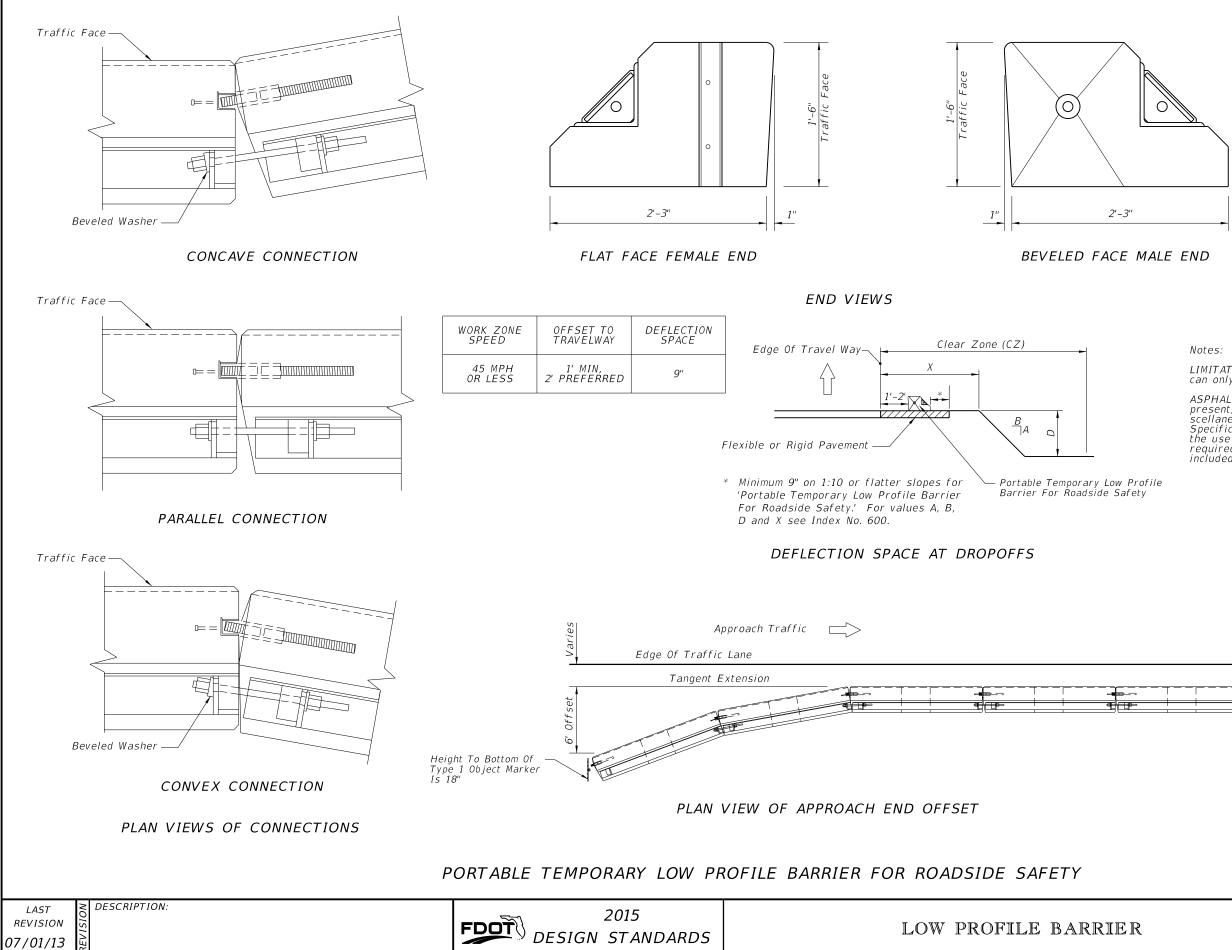
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		GENE	RAL NOTES
	1.		er For Roadside Safety' is a licensed design by nt on the rights of the designer shall be the sole
	2.	standard drawing is to indicate the appro	rtment and its assignees. The purpose for this val of use of the barrier on the State Highway r identifying the barrier unit; and, to provide
	3.	allowed for installation on the State High conform to Section 521 of the Standard a approved plants with quality control plan barrier wall unit shall be permanently ma the manufacturer, the producing precast permanent identification mark will serve	s licensed by the University Of Florida will be way System in Florida. Barrier wall units shall Specification and shall be produced in Department s for precasting concrete barrier walls. Each rked with an identification that is traceable to concrete plant and the date of production. This as certification that the unit has been ty of Florida drawings and specifications, and
	4.	by the licensed barrier producer. Units s interconnected segments in a run of barri	only with hardware and accessories furnished hall be used for no purpose other than as er. Low profile barrier wall units shall maintain tensioning rods shall be installed snug tight.
	5.	The low profile barrier is applicable for w	vork zone speeds of 45 mph or less.
	6.	ends and at 50' centers on tangents and to the top of the barrier by an adhesive Approach end units shall be marked with	and installed along the run of barrier at the 25' centers on radii. The markers shall be fixed or other method approved by the engineer. a Type I object marker. The cost of the tubular be included in the cost of the low profile barrier.
	7.	Information regarding licensing, shop dra certification of compliance can be obtaine Technology Licensing, P.O. Box 115500, Ga 352–392-8929, Fax: 352–392-6600. Refe	l from the University Of Florida: Office of nesville, Florida, 32611-5500. Telephone:
	8.		er For Roadside Safety shall be paid for under emporary) Low Profile Concrete, LF, and will be g, maintaining and removing barrier wall.
	9.	Deflection space shall be kept clear of a equipment, and objects.	ny grass, construction debris, stockpiled materials,
Unit Length 12 or	TAB		END PICTORIAL VIEWS ILE BARRIER FOR ROADSIDE SAFETY
LAST S DESCRIPTION:		<del>5</del> 71 2015	
REVISION IS D7/01/14	FD	DESIGN STANDARDS	LOW PROFILE BARRIE

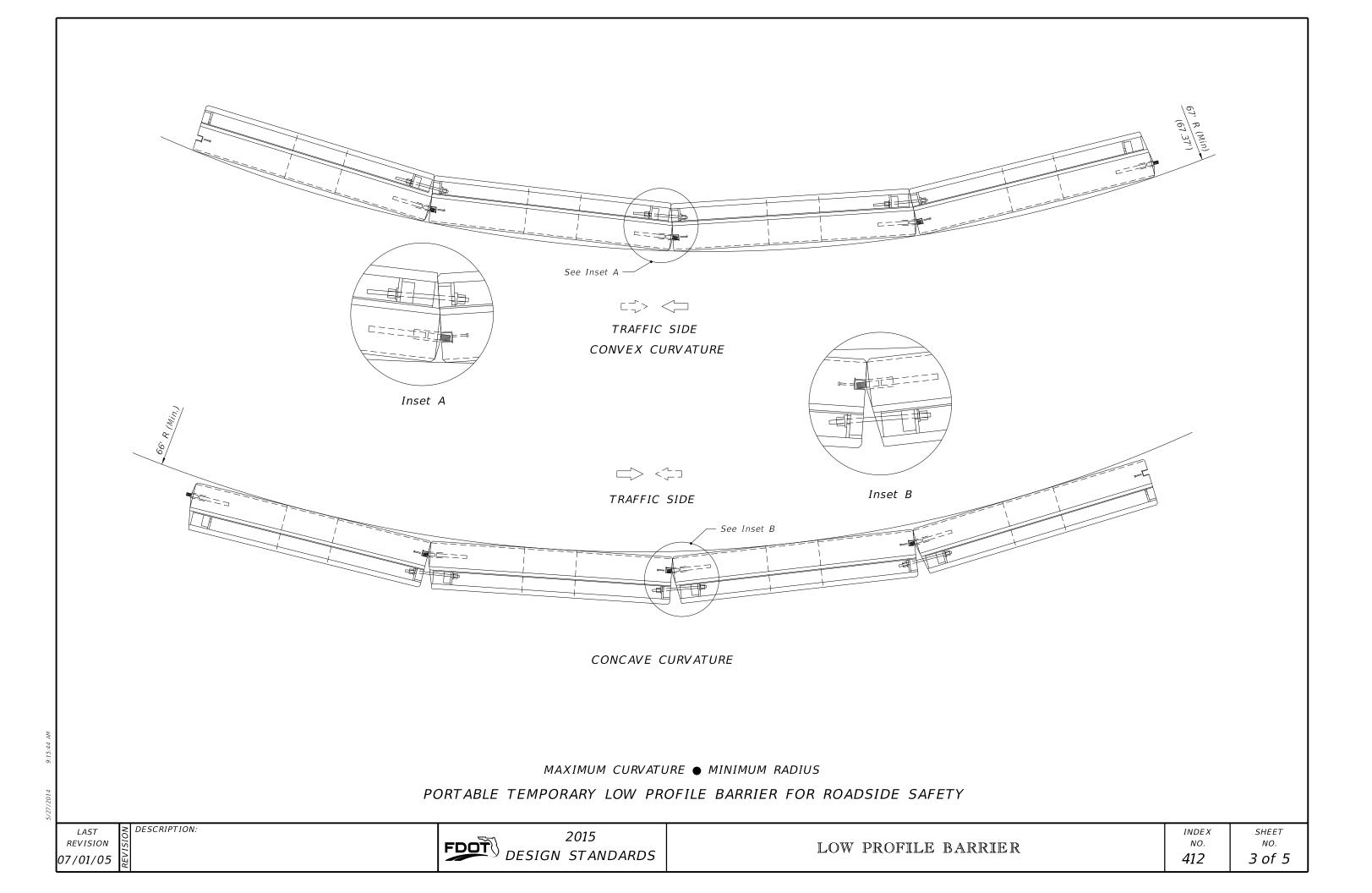


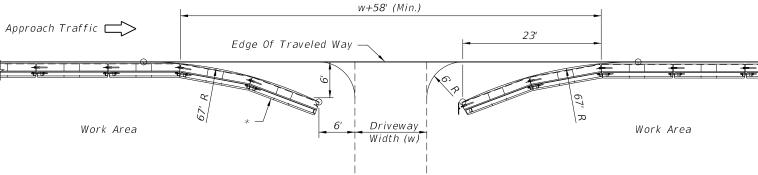


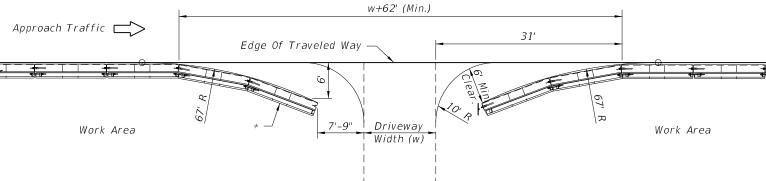
LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement.

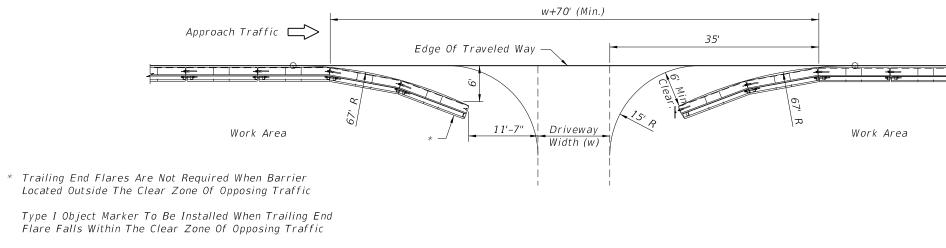
ASPHALT PAD: Where exisiting pavement is not present, construct 2" Asphalt Pad using mi scellaneous asphalt pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. Payment for asphalt pad will be included in the cost of the barrier.

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R		INDEX NO.	SHEET NO.
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# LEGEND

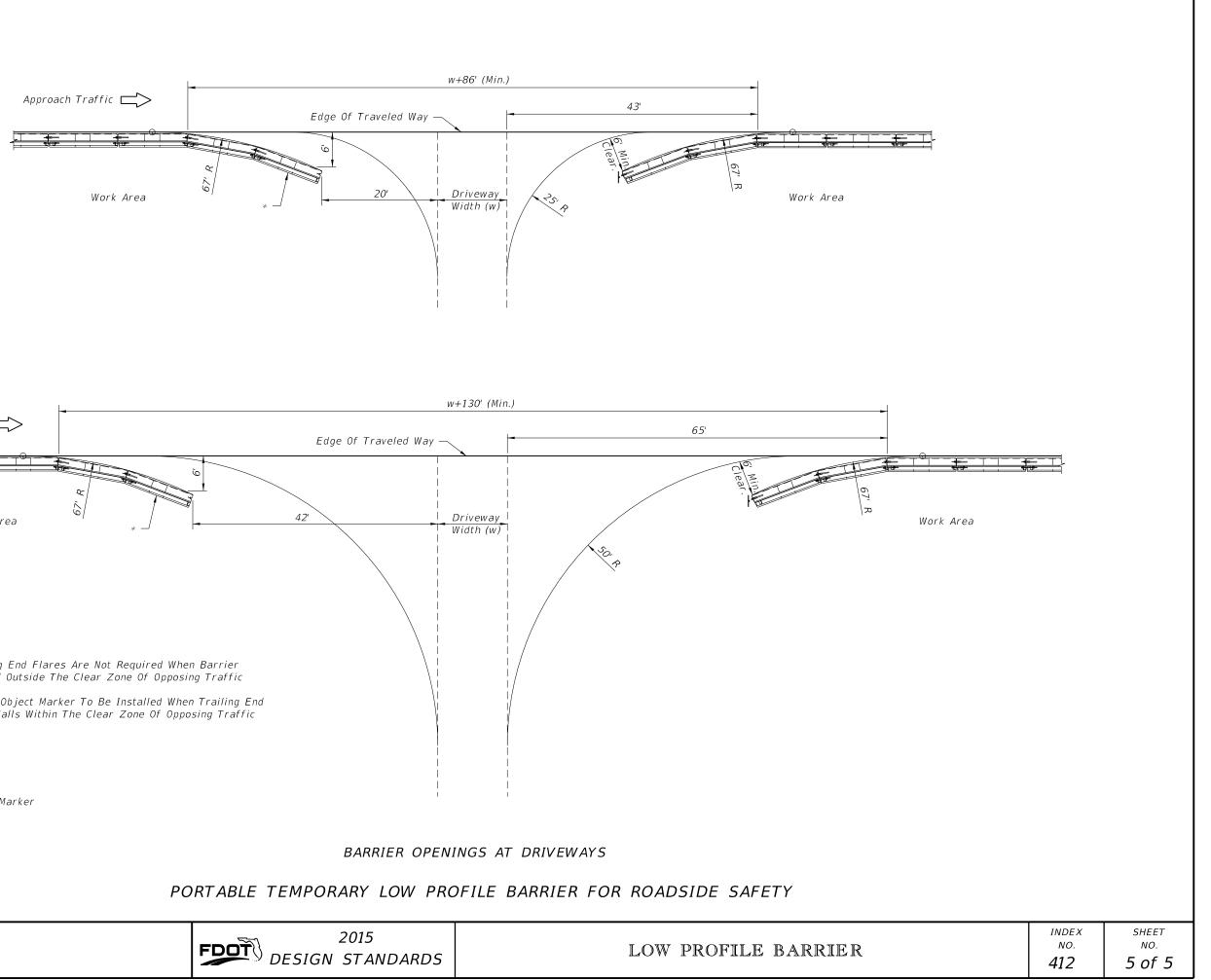
Type I Object Marker

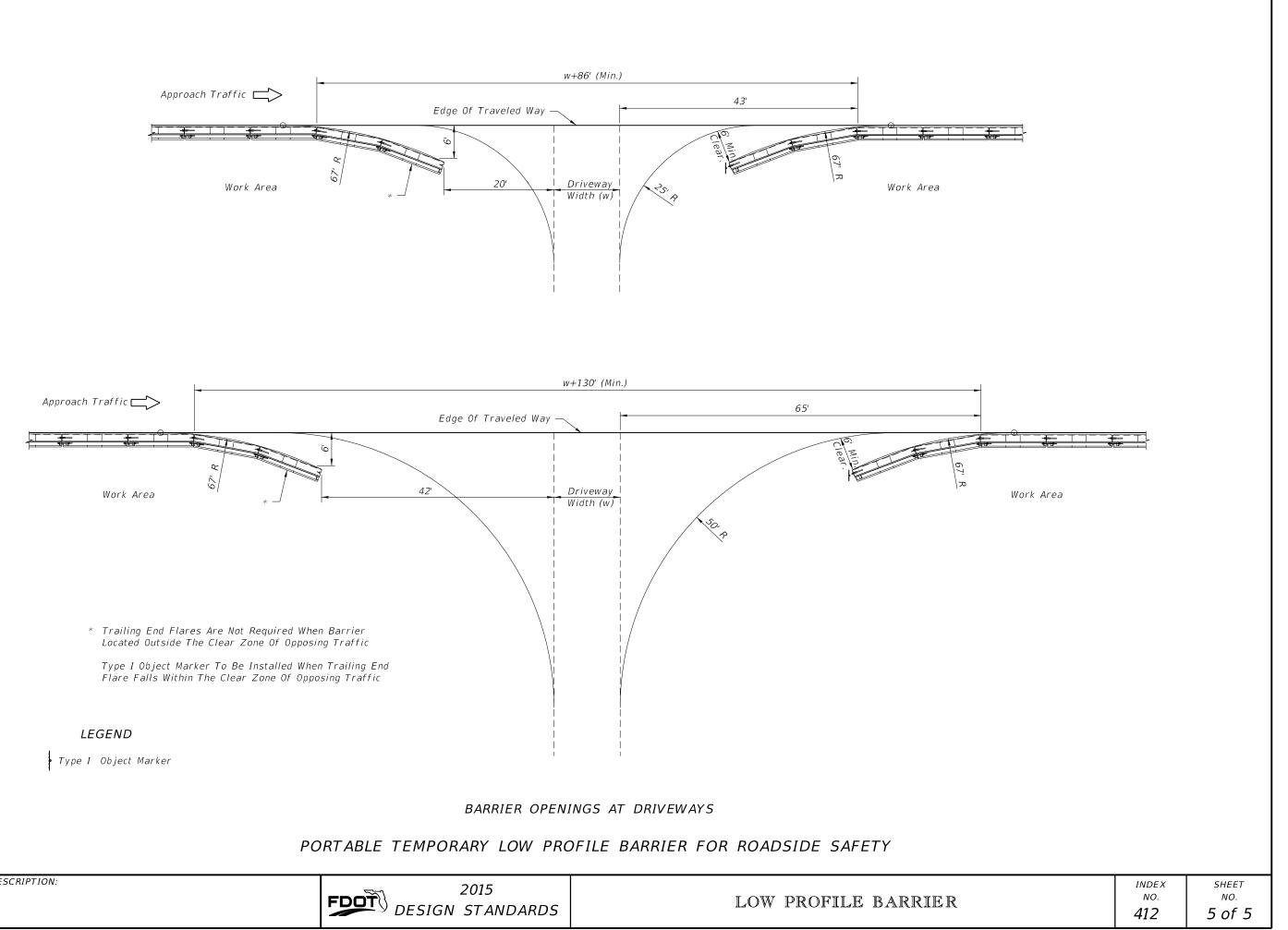
BARRIER OPENINGS AT DRIVEWAYS

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

LAST REVISION	DESCRIPTION:		LOW PROFILE BARRIE
01/01/12		DESIGN STANDARDS	

INDEX SHEET NO. NO.				
TR INDEX SHEET NO.				
INDEX SHEET NO. NO.				
INDEX SHEET NO. NO.				
INDEX SHEET NO. NO.				
NO. NO.				
NO. NO.				
	R	index no. <b>412</b>	sheet NO. <b>4 of 5</b>	





	DESCRIPTION:	2015 <b>FDOT</b> DESIGN STANDARDS	
01/01/12			

The Type K Temporary Concrete Barrier System has been crash tested to NCHRP Report 350 TL-3 criteria or structurally evaluated to meet the requirements of NCHRP Report 350 TL-3 criteria for the installation configurations as shown utilizing the types, sizes, lengths, shapes, strengths and grades of the fabrication and installation materials as shown.

In order to maintain crashworthiness of the system, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

### FABRICATION NOTES:

- FABRICATOR PREQUALIFICATION: The Barrier Units shall be made in a prestressed concrete plant that meets the requirements of Specification Section 450 or in a precast plant meeting the requirements of Specification Section 105.
- CONCRETE: Concrete shall be Class IV in accordance with Specification Section 346. Specification Sections 346-10.2 through 346-10.4 are not applicable. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.
- REINFORCING STEEL: All reinforcing steel shall be ASTM A 615, Grade 60 except for Bars 6D1, 6D2 and 6D3. Bars 6D1, 6D2 and 6D3 shall be ASTM A 706 except that a 2<sup>3</sup>/<sub>4</sub>" diameter pin must be used for the 180 degree bend test. After fabrication, all or part of Bars 6D shall be hot dip galvanized in accordance with Specification Section 962 or coated with a cold galvanizing compound in accordance with Specification Section 975. The minimum limit of galvanizing or coating is shown in the Bending Diagrams. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated. Install Bars 6D within  $\frac{1}{8}$ " of the plan dimensions. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.

At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with Specification Section 931 and the details shown on Sheet may be utilized in lieu of Bars 4A and 5B.

All dimensions in the Bending Diagrams are out to out. All reinforcing steel shall have a 2" minimum cover except as noted.

- LIFTING SLEEVE ASSEMBLY: Inclusion of the Lifting Sleeve Assemblies is optional. Steel for Pipe Sleeve shall be in accordance with ASTM A 53. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.
- SURFACE FINISH: Construct Barrier Units in accordance with Specification Sections 400 and 521. Finish the top and sides of the Barrier Units with a General Surface Finish. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish. Use stationary metal forms or stationary timber forms with a form liner.

MARKING: Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall. Ink stamps are not allowed. Permanently mark with the following information:

- Туре К1
- Eabricator's name or symbol
- Date of manufacture (day, month and year)

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

Alternate Designs: Manufacturers/vendors seeking approval of proprietary Temporary Barrier Systems for inclusion on the Approved Products List (APL) as alternative designs shall submit a Product Application package. The application package shall include manufacturer's product drawings, specifications, installation manual, National Cooperative Highway Research Program (NCHRP) Report 350 or Manual for Assessing Safety Hardware (MASH) Test Level 3 (TL-3) crash test documentation and the FHWA "Letter of Acceptance." The posted APL drawings will need to include the following:

1. Anchorage, bolting, and staking details for connections to asphalt and concrete pavement.

- 2. Sections and tables showing required deflection space and minimum offsets to above ground hazards or drop-offs.
- 3. Alignment and length of need details.
- 4. Transition and overlap details.
- 5. End treatment details.

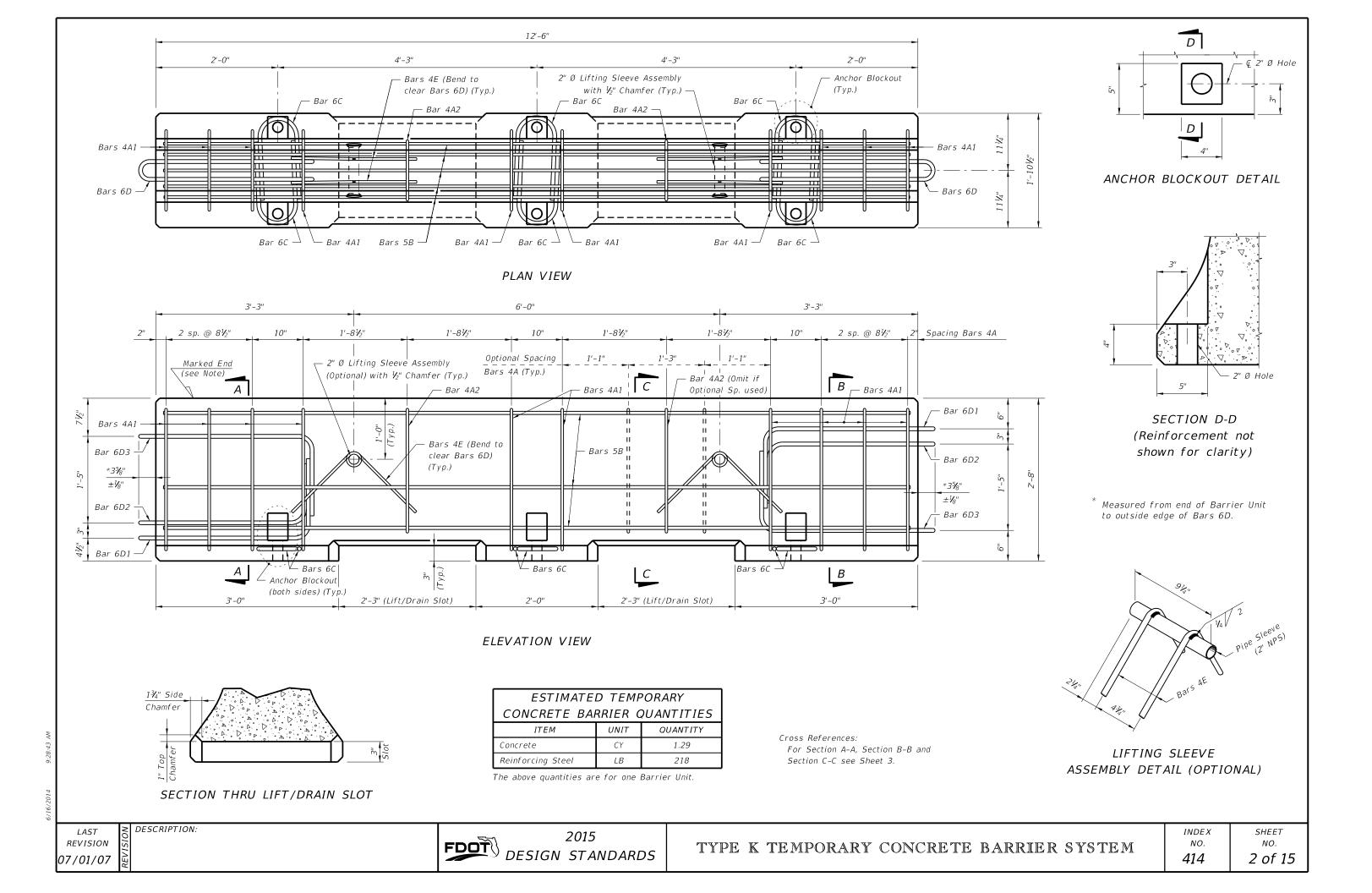
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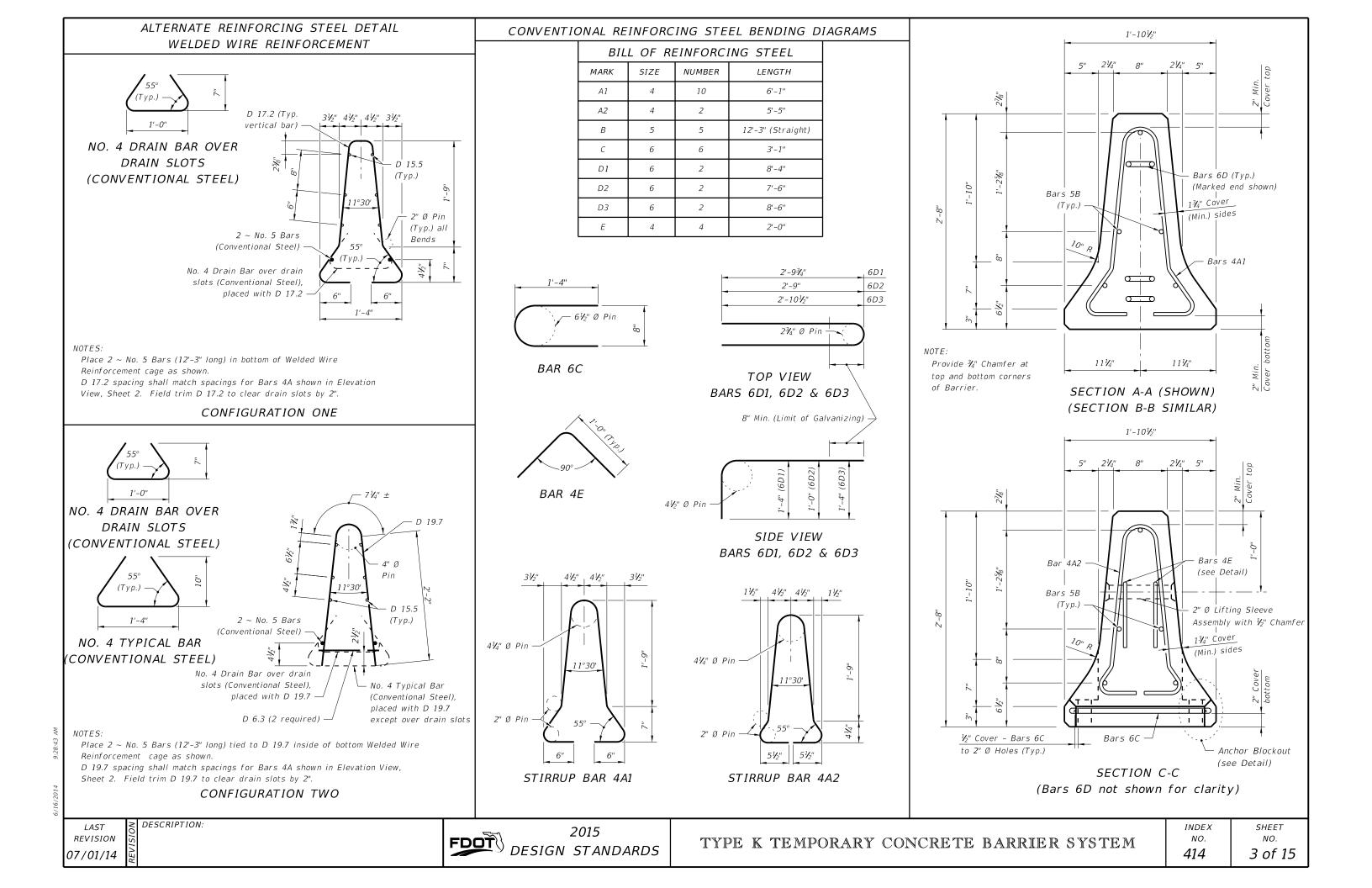
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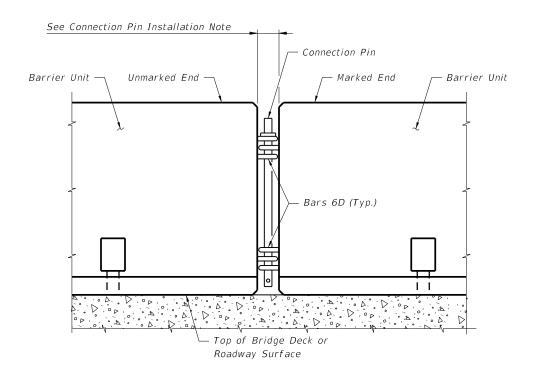
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TYPE K TEMPORARY CONCRETE BA

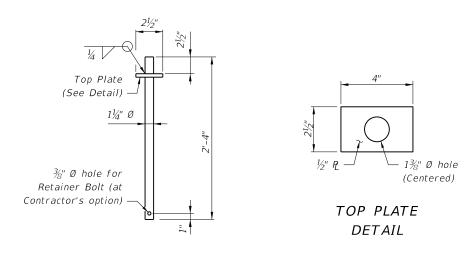
RRIER SYSTEM	INDEX NO.	SHEET NO.
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DETAIL OF CONNECTION BETWEEN BARRIER UNITS



CONNECTION PIN DETAIL

### NOTES FOR ALL INSTALLATIONS:

- 1. LIMITATION OF USE: This Temporary Concrete Barrier System is intended for work zone traffic control and other temporary applications. It shall not be used for permanent traffic railing construction unless specifically permitted by the Plans. Except as shown for the Back Filled Roadway Installations, the Barrier Units must be installed on a flexible pavement (asphalt) or rigid pavement (concrete) surface as shown with a cross slope of 1:10 or flatter. Except as shown for transition installations, Type K Barrier Units are not intended to be bolted down or staked down in locations where they can be impacted from the back side.
- 2. HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.
- 3. SURFACE PREPARATION: Except as shown for the Back Filled Roadway Installations, remove all grass, debris, loose dirt and sand from the pavement, bridge deck or Asphalt Pad surface within the barrier footprint just prior to placement of the Barrier Units.
- 4. OFFSET TO TRAVELWAY: Offset shall meet requirements as shown on sheet 1 of Index 415. 5. CONNECTION PIN ASSEMBLY: Steel for Connection Pin and Top Plate assemblies shall be in accordance with ASTM A36 or ASTM A709 Grade 36.
- Nondestructive testing of welds shall not be required. At the Contractor's option, a 🔏 diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.
- 6. CONNECTION PIN INSTALLATION: Initially set Barrier Units by using a  $3\frac{5}{8}$ " wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 5). Barrier Units shall not be used unconnected.
- 7. DELINEATION: Mount Barrier Delineators on top of Barrier Units that are used as traffic barriers along travel ways in work zones. Space the Barrier Delineators at 50' centers in alignment transitions, 100' centers on horizontal curves and 200' centers on tangent alignments.
- 8. REUSE OF UNITS: Barrier Units may be reused provided they have the structural integrity and surface qualities of new units. Do not use Barrier Units without Marking Plates.
- 9. MAINTENANCE: Deflection space shall be kept clear of any grass, construction debris, stockpiled materials, equipment, and objects.
- 10. REUSE OF CONNECTION PINS: Connection pins may be reused if they have the structural integrity of new pins.
- 11. INSTALLATIONS ON CURVED ALIGNMENTS: The details presented in these Standards are shown for installations on tangent alignments. Details for horizontally curved alignments are similar.
- 12. TRANSITIONS: Transitions are required between freestanding, bolted down, staked down and back filled Type K Barrier installations, see Sheet 8 for transition requirements and details. Transitions are also required between installations of Type K Barrier and other types of temporary barrier, see Index No. 415 for transition requirements and details. Splices and transitions are required between installations of Type K Barrier and permanent Bridge or Roadway Traffic Railings, see Sheets 9 through 13 for transition requirements and details. Transitions are required between installations of Type K Barrier and Proprietary (APL) Barrier Systems, See Sheets 14 and 15 for transition requirements and details.
- 13. PAYMENT: Barrier Units for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary) (F&I) (Type K), LF. Any relocation of the Barrier Units required for the project shall be paid for under the contract unit price for Barrier Wall (Temporary) (Relocate) (Type K), LF. The Contractor shall furnish Barrier Units except when the Plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return. Unless otherwise noted on the Plans, the Barrier Units shall become the property of the Contractor and shall be removed from the site prior to acceptance of the completed project.

### NOTES FOR THRIE BEAM GUARDRAIL SPLICE INSTALLATIONS:

- 1. THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices in accordance with AASHTO M 180, Type II (Zinc coated) and as follows: Two panels per splice (One panel per side) of Class B (10 Gauge), or
  - Four panels per splice (Two nested panels per side) of Class A (12 Gauge).

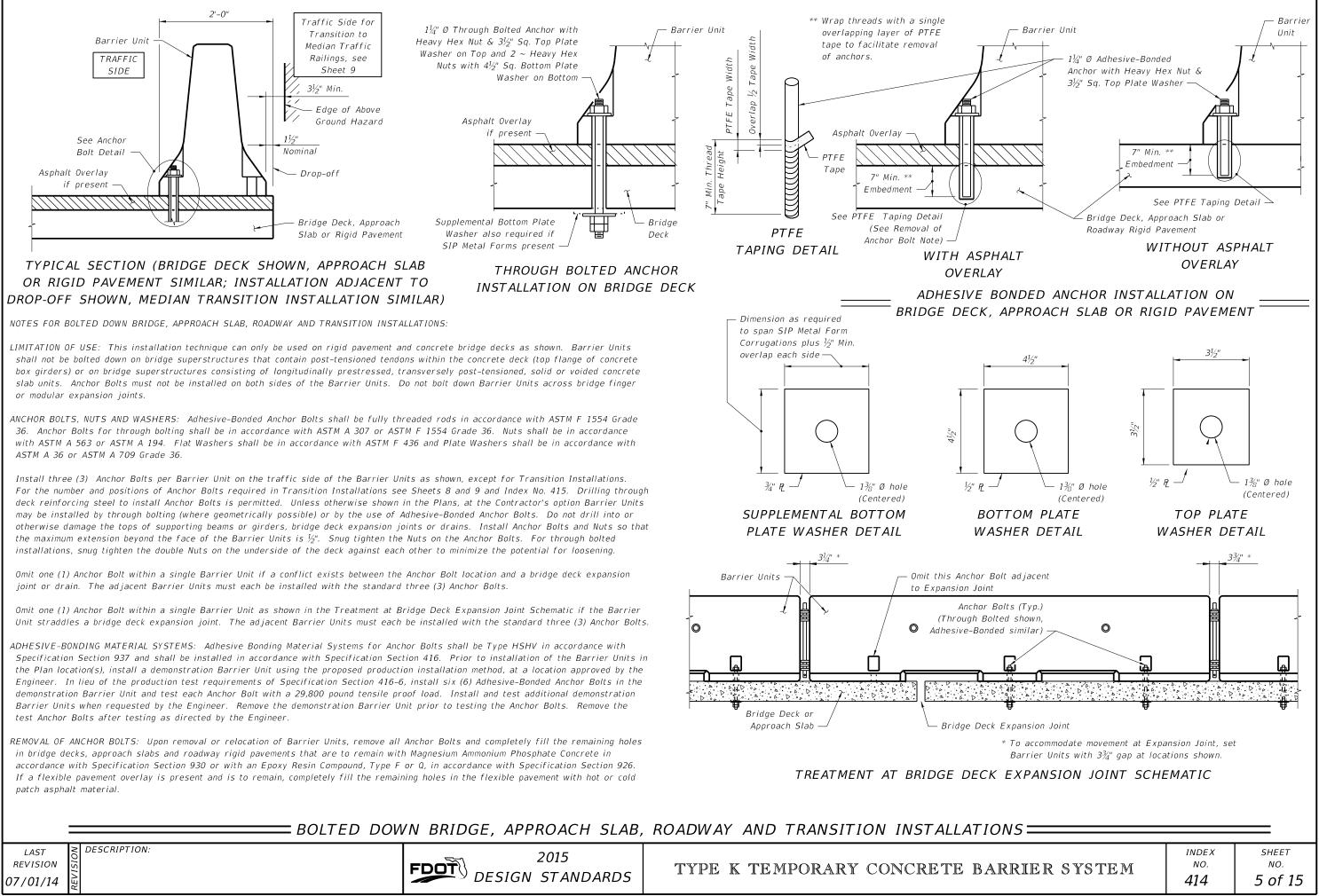
Guardrail panel length shall be 12'-6". Provide and install all other associated metallic guardrail components (Terminal Connectors, Shoulder Bolts, Hex Bolts and Nuts, Filler Plates, etc.) in accordance with Index No. 400.

Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted. Do not drill or cut through utilities or conduits within permanent concrete traffic railings.

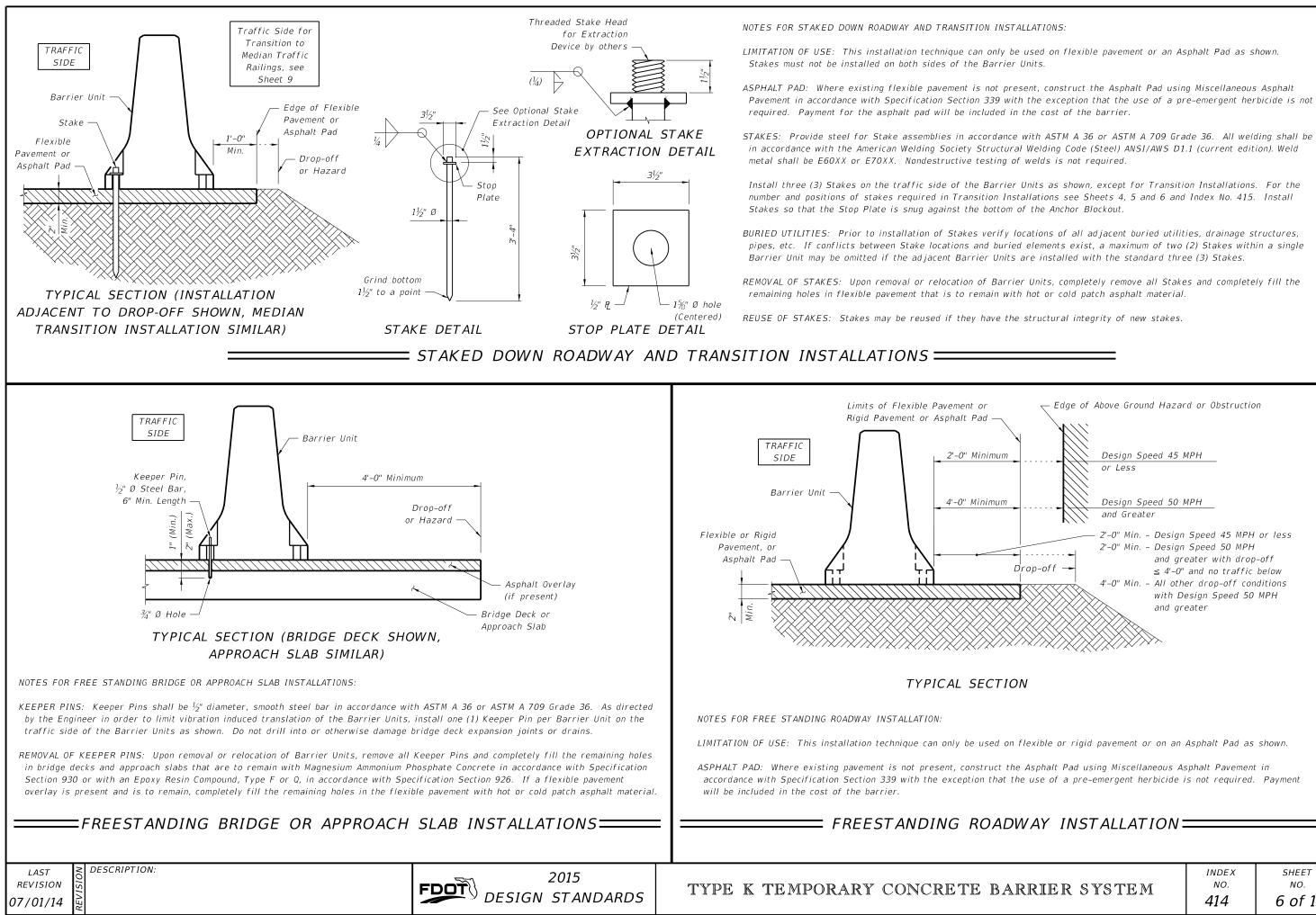
- 2. GUARDRAIL OFFSET BLOCKS: Provide and install timber Offset Blocks meeting the material requirements of Index No. 400. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.
- 3. CONCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification Section 346, any Class, or a commercially available prebagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification Section 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.

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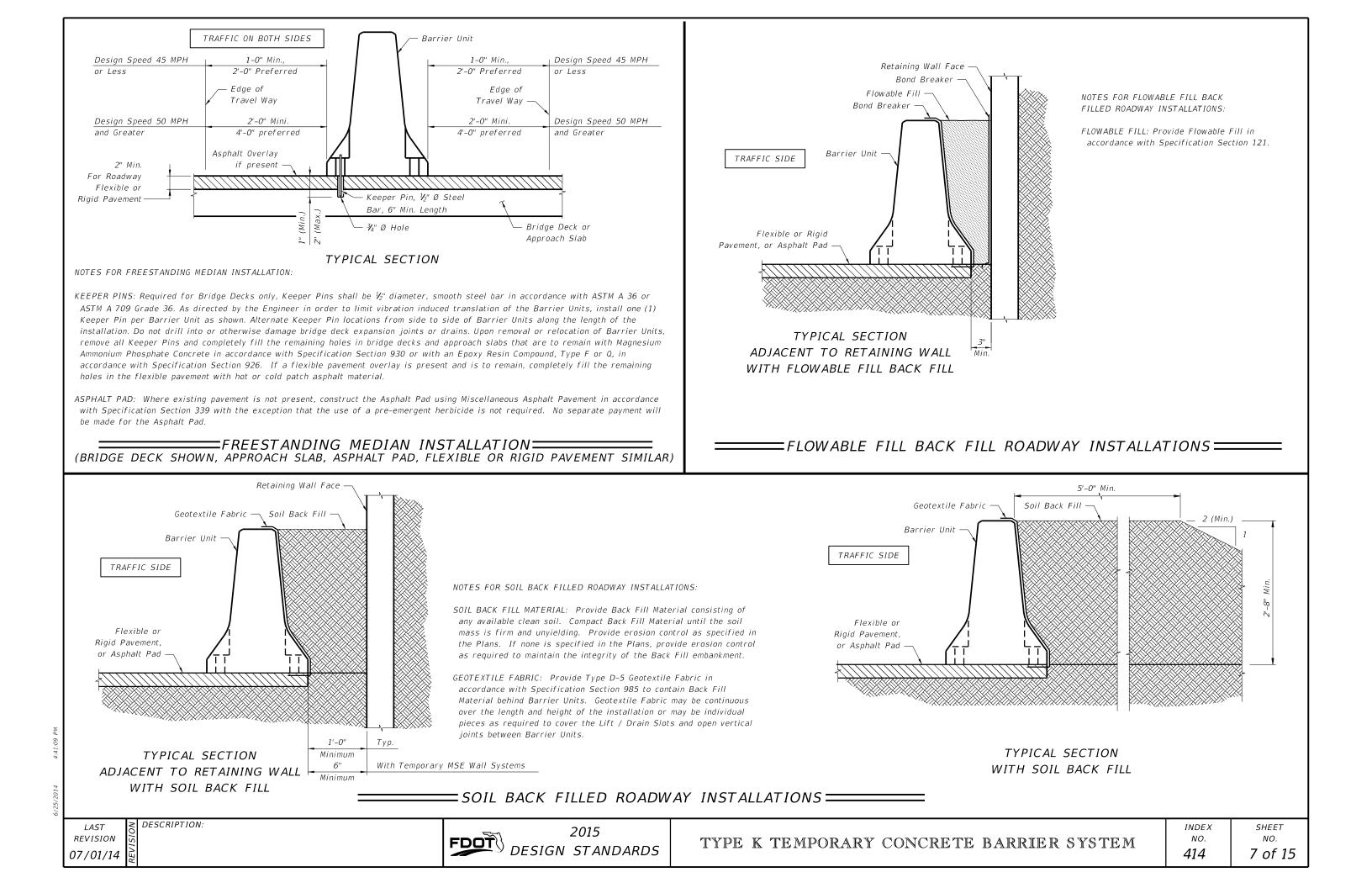
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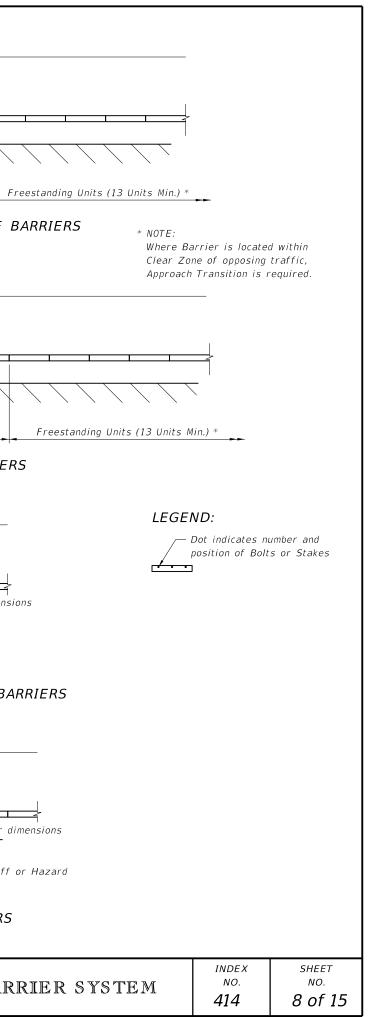
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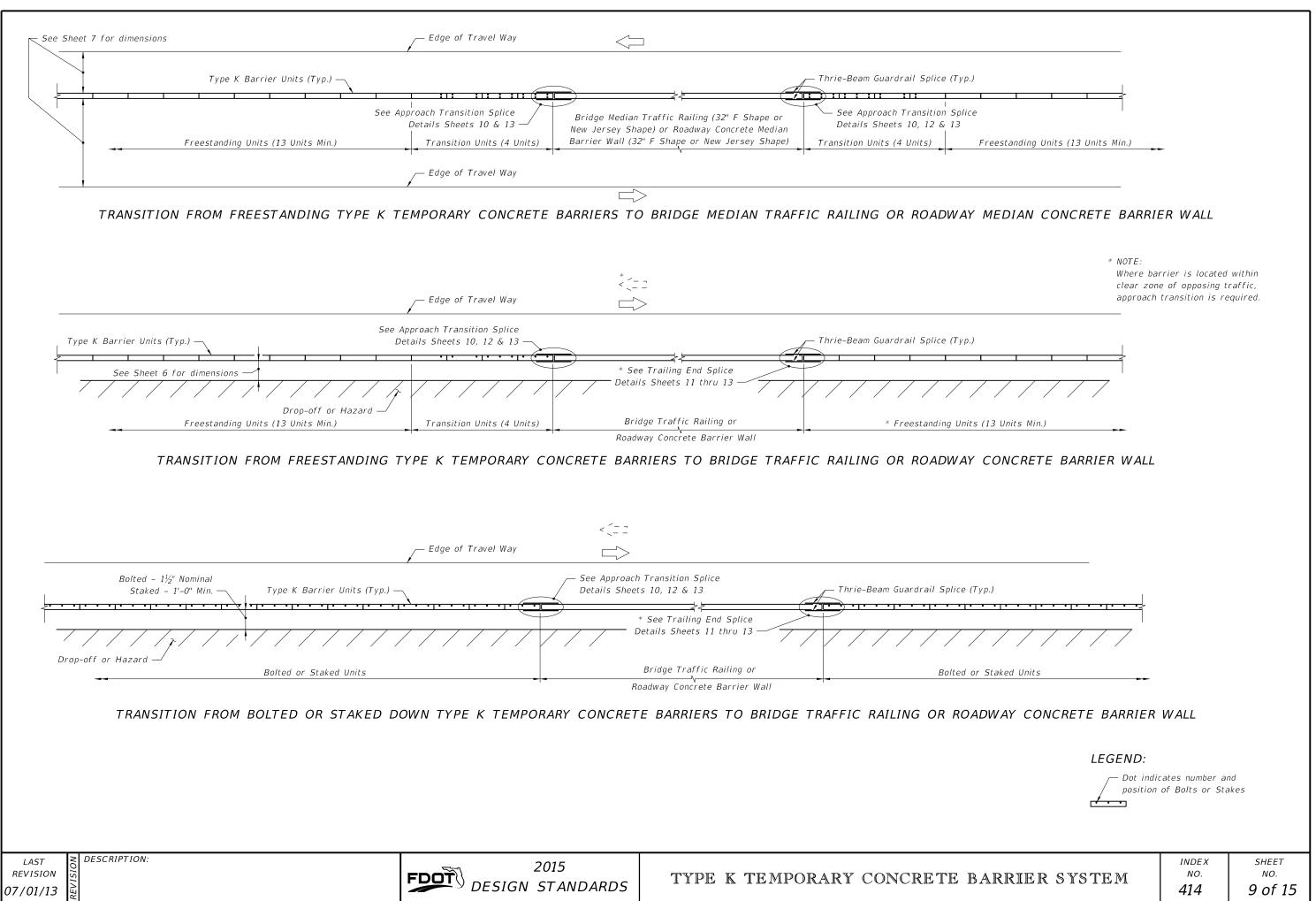


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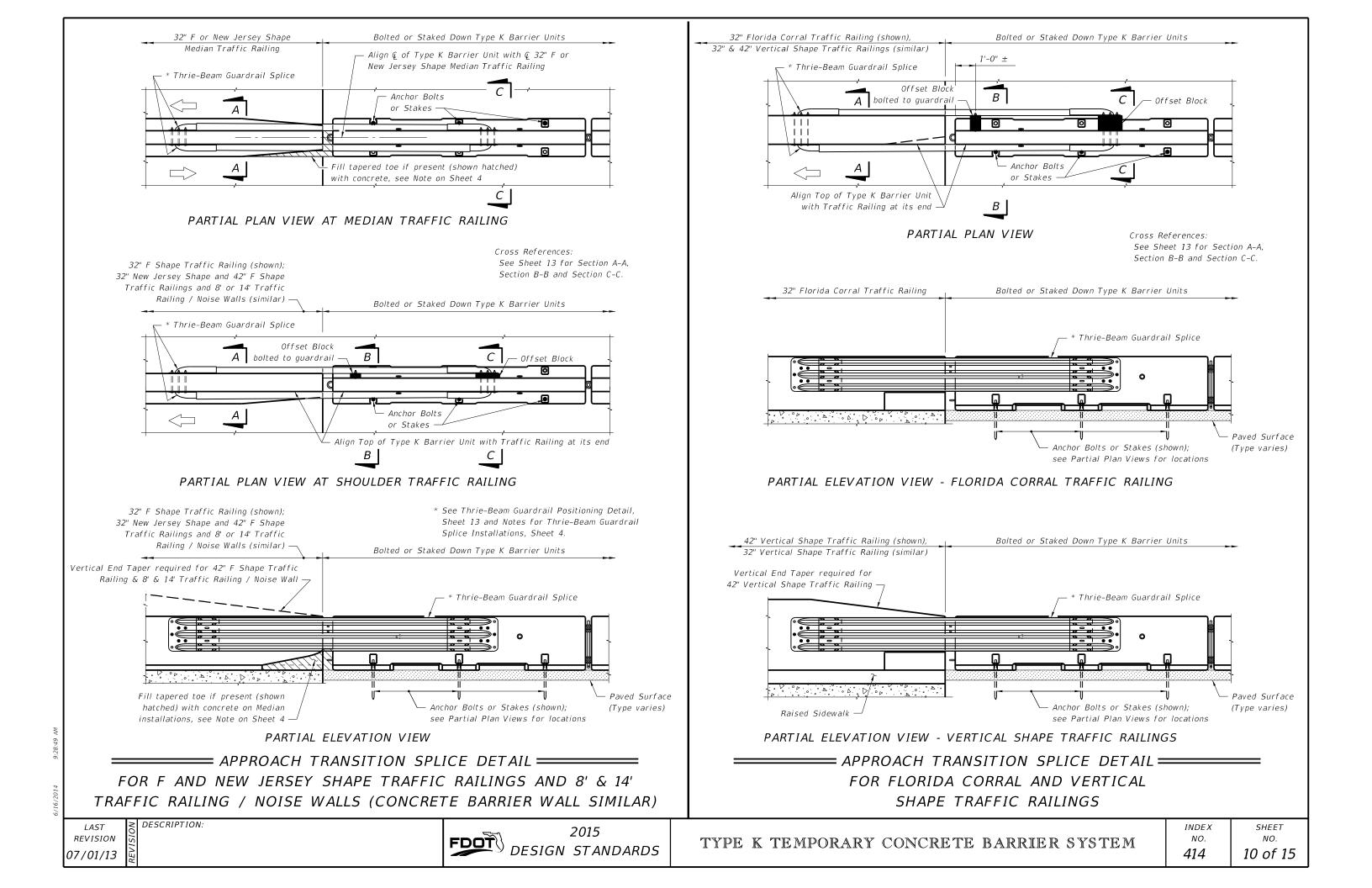


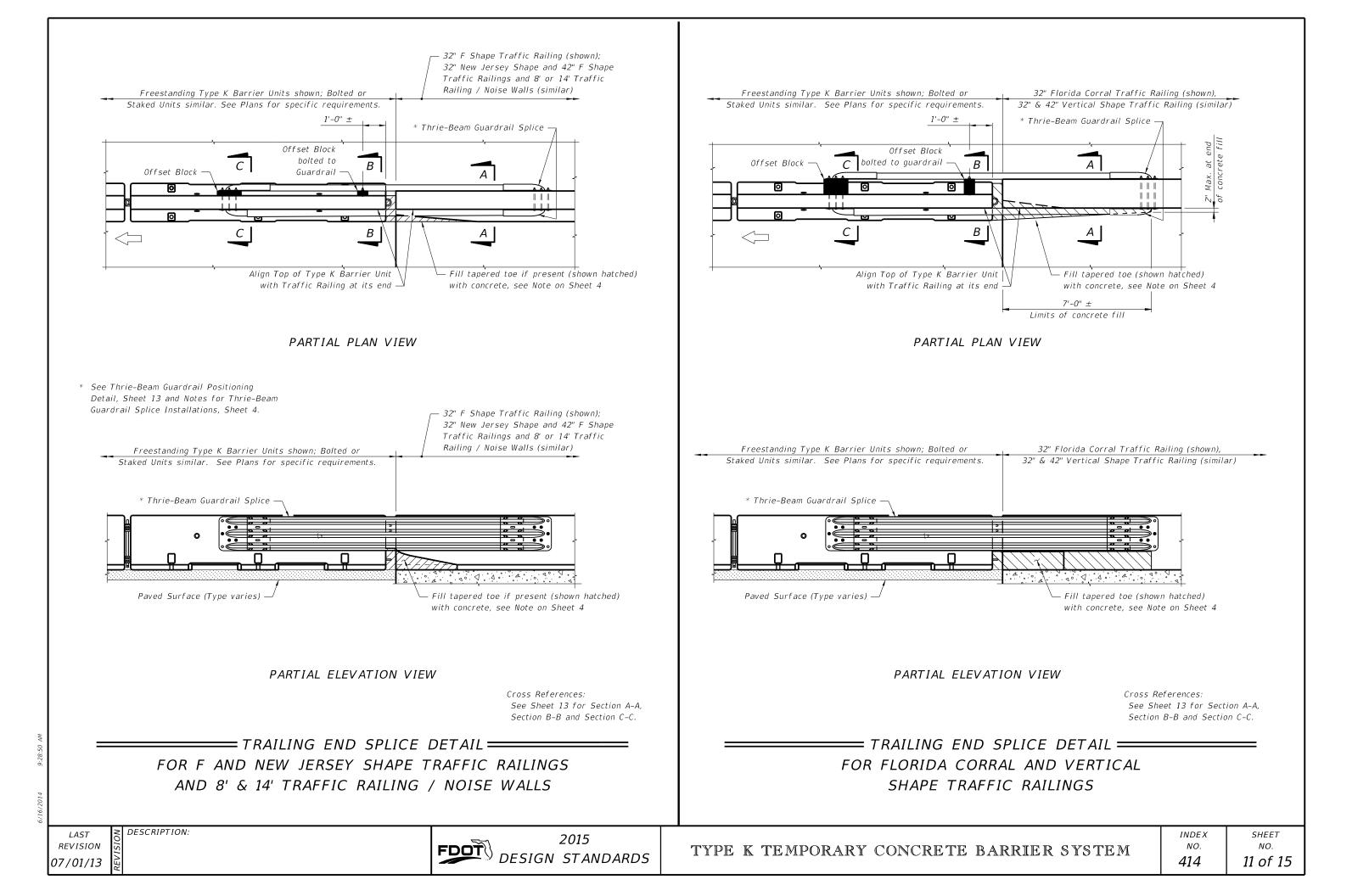
		*<	
			Edge of Travel Way
<u></u>	Type K Barrier Units (Typ.)	First full Barrier Unit before Drop-off or Hazard shielded by Bolted or Staked Units	
See Sheet 6 for dimensions	///////////////////////////////////////	Drop-off or Hazard	
Freestanding Units (13 Units N	Min.) Transition Units (4 Units)	Bolted or Staked Units	Transition Units (4 Units) *
APPROACH TRA	NSITION FROM FREESTANDING TO	BOLTED OR STAKED DOWN	TYPE K TEMPORARY CONCRETE
		* <	Edge of Travel Way —
	Type K Barrier Units (Typ.) — First full B	arrier Unit before Back Filled Units	
See Sheet 6 for dimensions -			
Drop-off or Hazard		Back Fill —	
Freestanding Units (13 Units Min.)	) Transition Units (4 Units)	Back Filled Units	Transition Units (4 Units) *
 	First full Barrier Unit after Drop-off or Hazard shielded by Bolted or Staked Units	— Type K Barrier Units	(Typ.)
Bolted – 1½" Nominal Staked – 1'-0" Min.			See Sheet 6 for dimen
	Bolted or Staked Units	Freestanding	Units
TRAILING END TRANS	SITION FROM BOLTED OR STAKED	DOWN TO FREESTANDING T	PE K TEMPORARY CONCRETE B
			Edge of Travel Way —
		— Type K Barrier Units	(Тур.)
			See Sheet 6 for
	Back Fill —		Drop-of
	Back Filled Units	<b>*</b>   <b>*</b>	
	D TRANSITION FROM BACK FILLED	I U FREESIANDING ITPE K I	EMPORARI CONCRETE BARRIER
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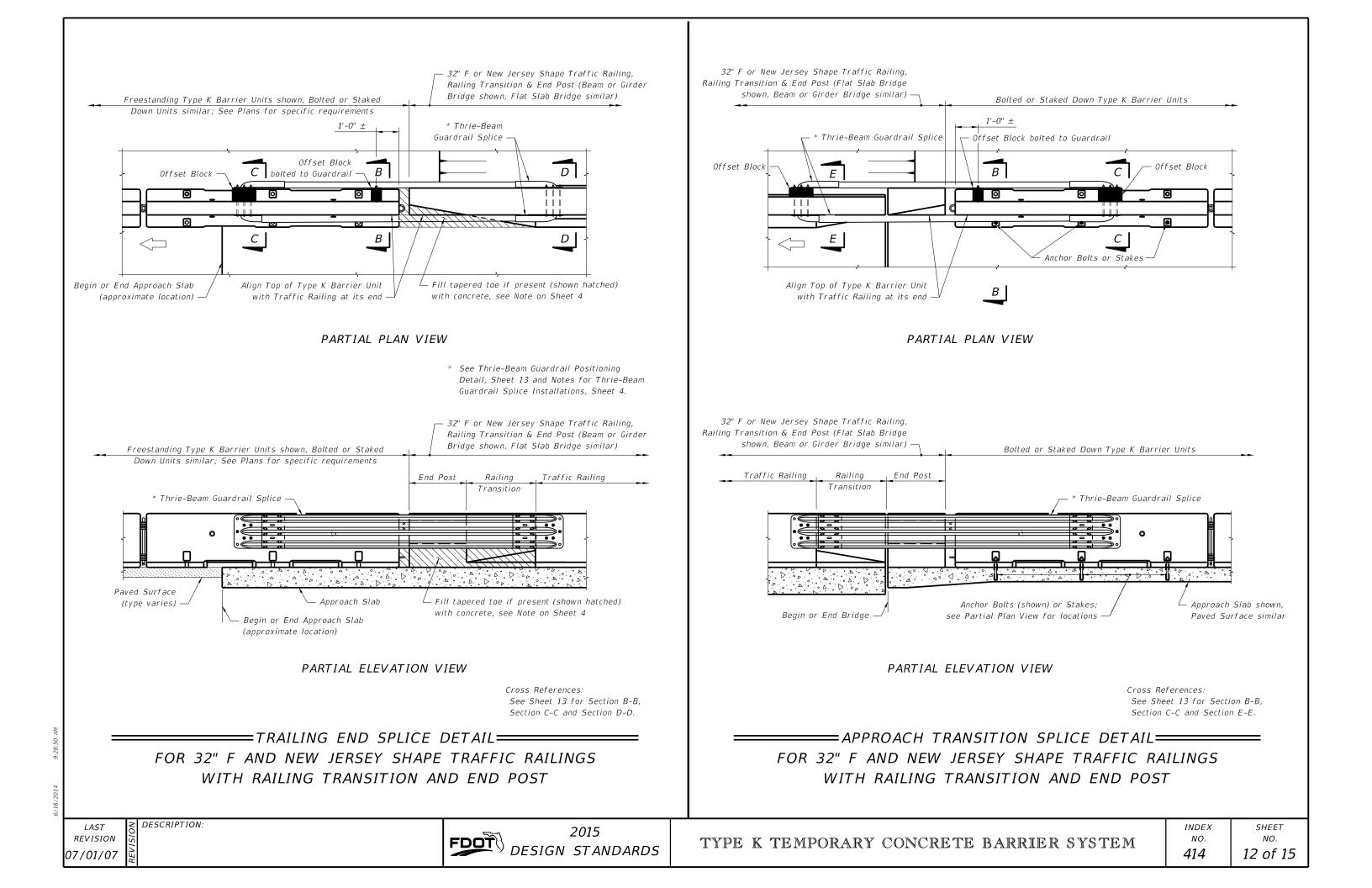


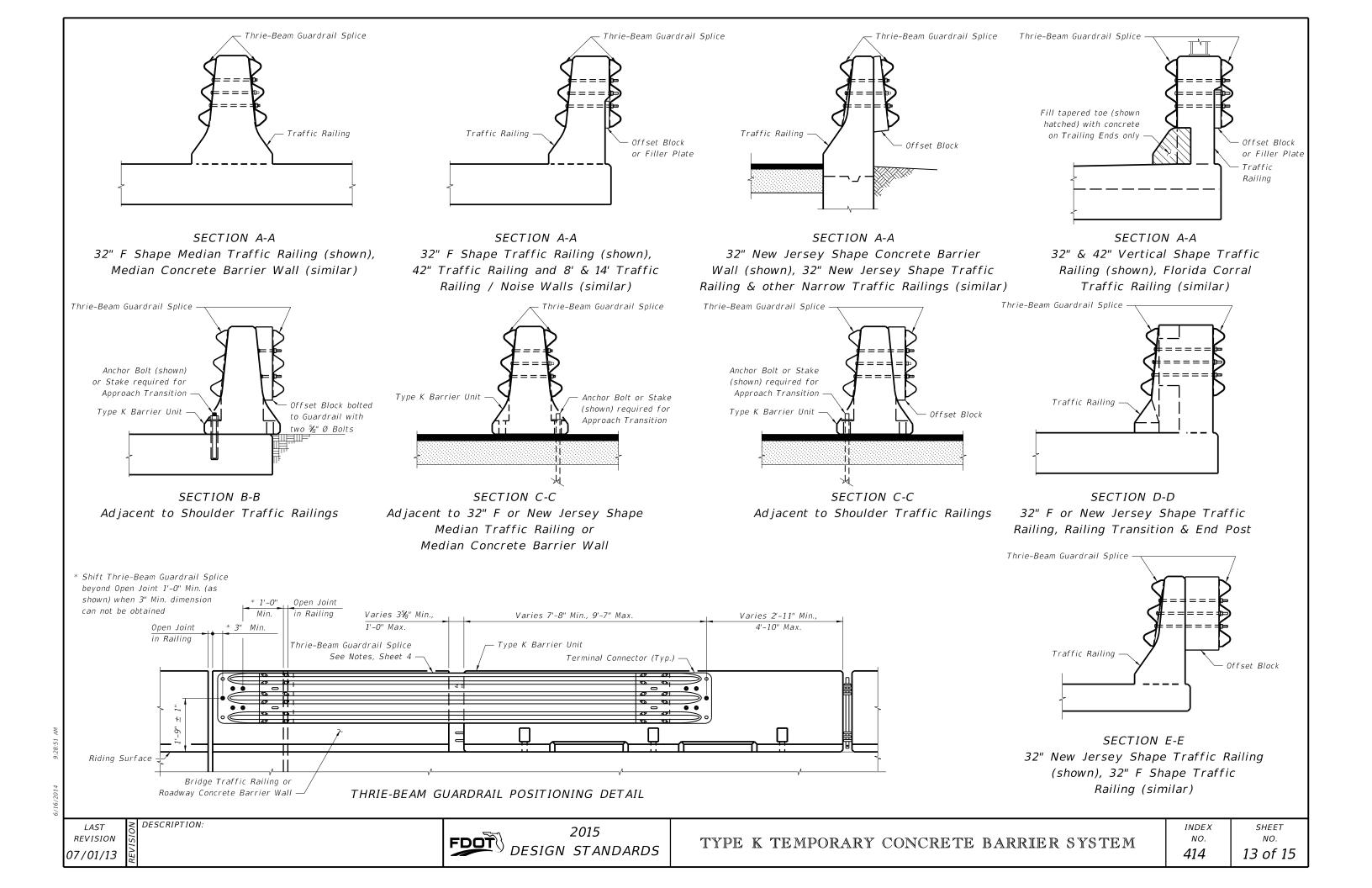


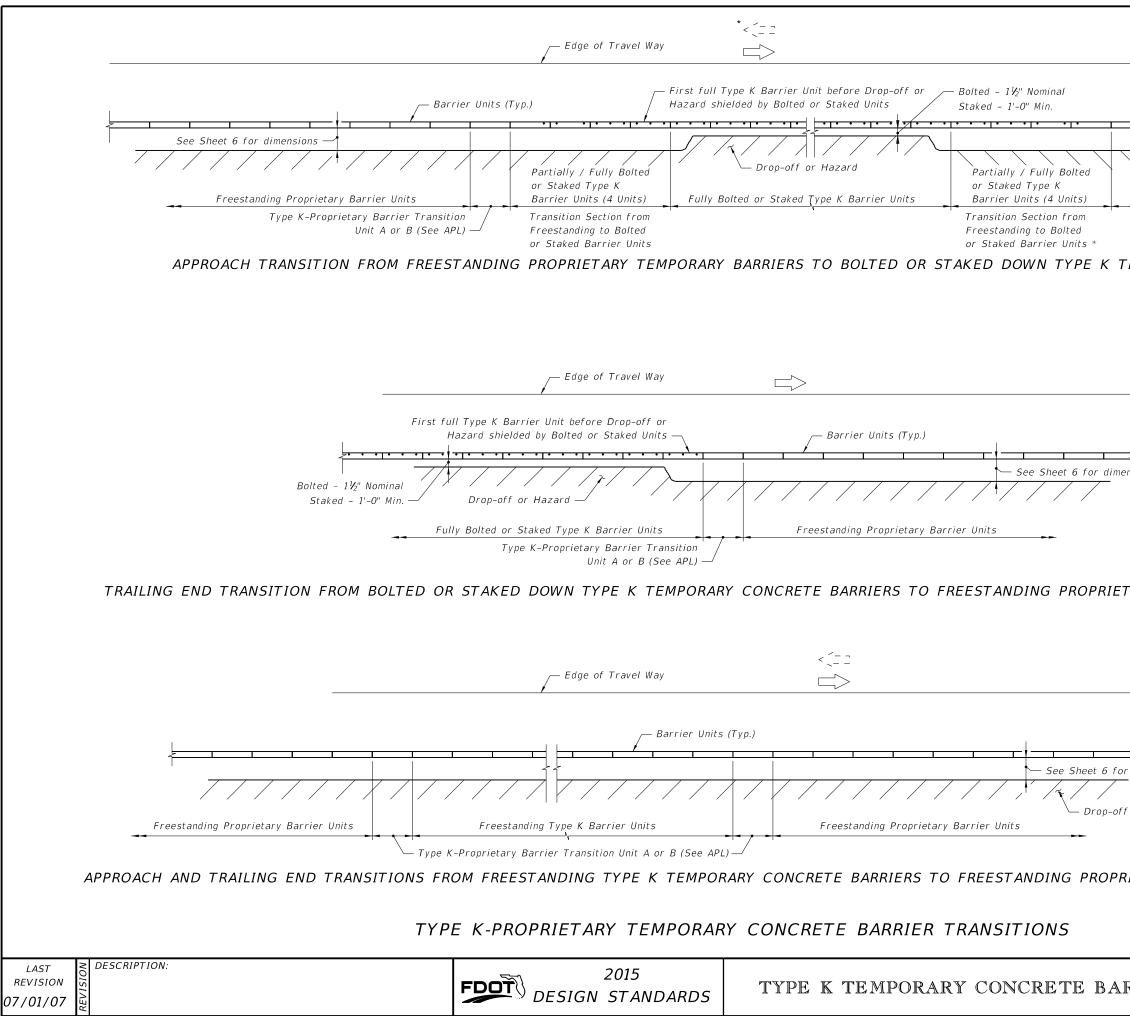
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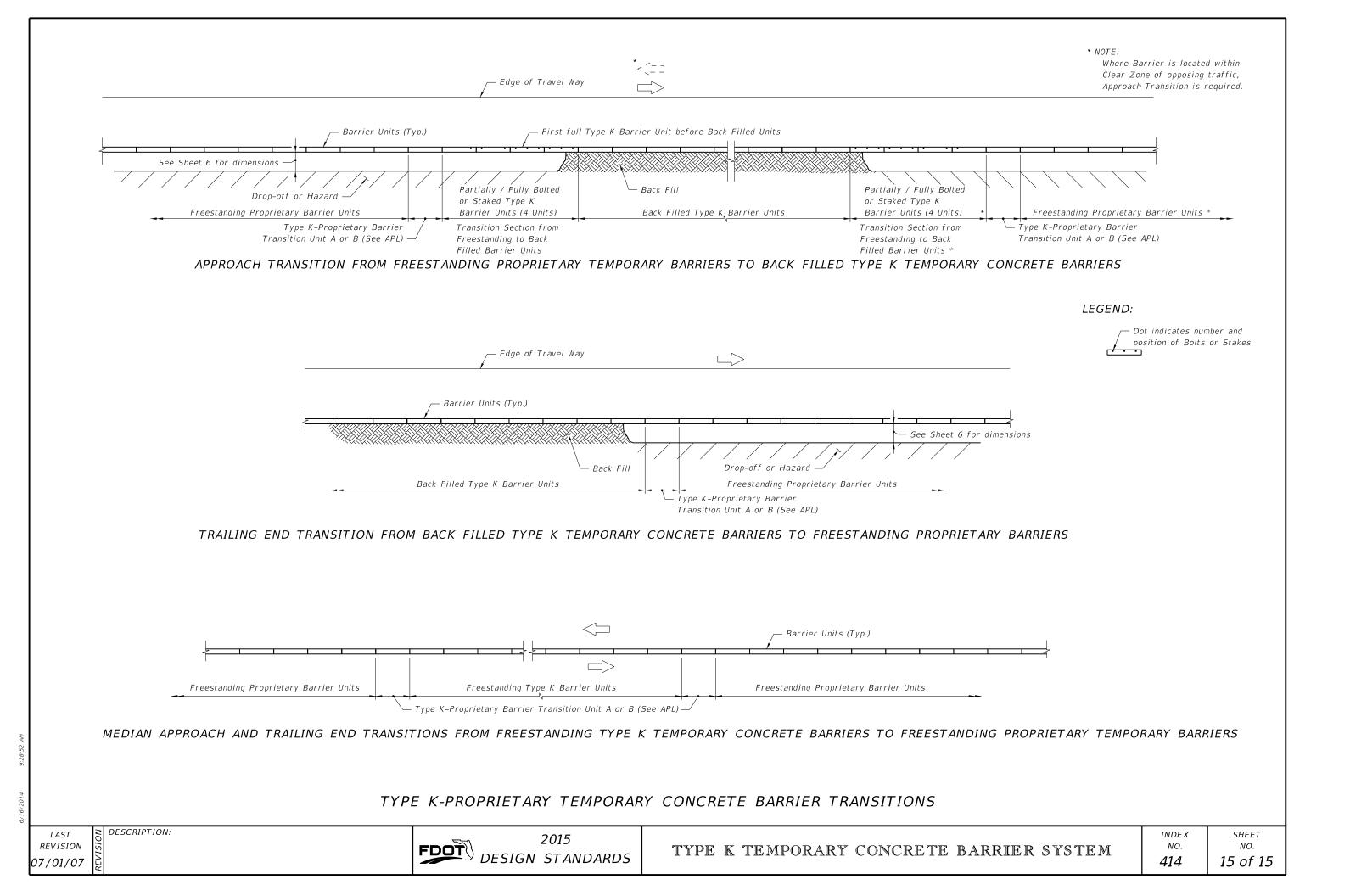








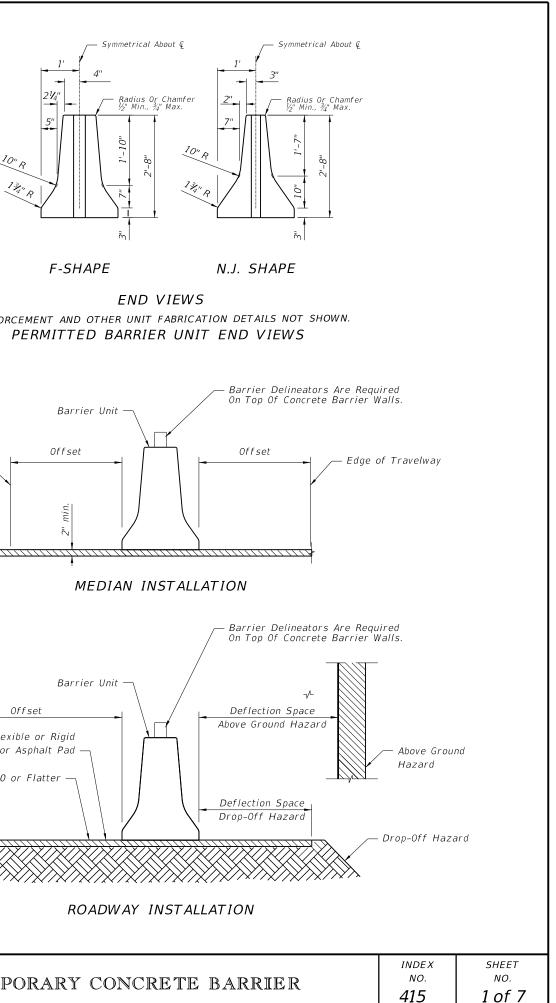
Clear Zone	rier is located of opposing tr ransition is re	affic,	
	-		
Freestanding Proprietary E Type K-Proprietary Barrier Tra Unit A or B (See APL)			
EMPORARY CONCRETE B	ARRIERS		
LEGEND:	Dot indicates n	umber and	
	position of Bolt		
nsions			
TARY TEMPORARY BARRIE	ĒRS		
r dimensions			
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RIETARY TEMPORARY BAR	RIERS		
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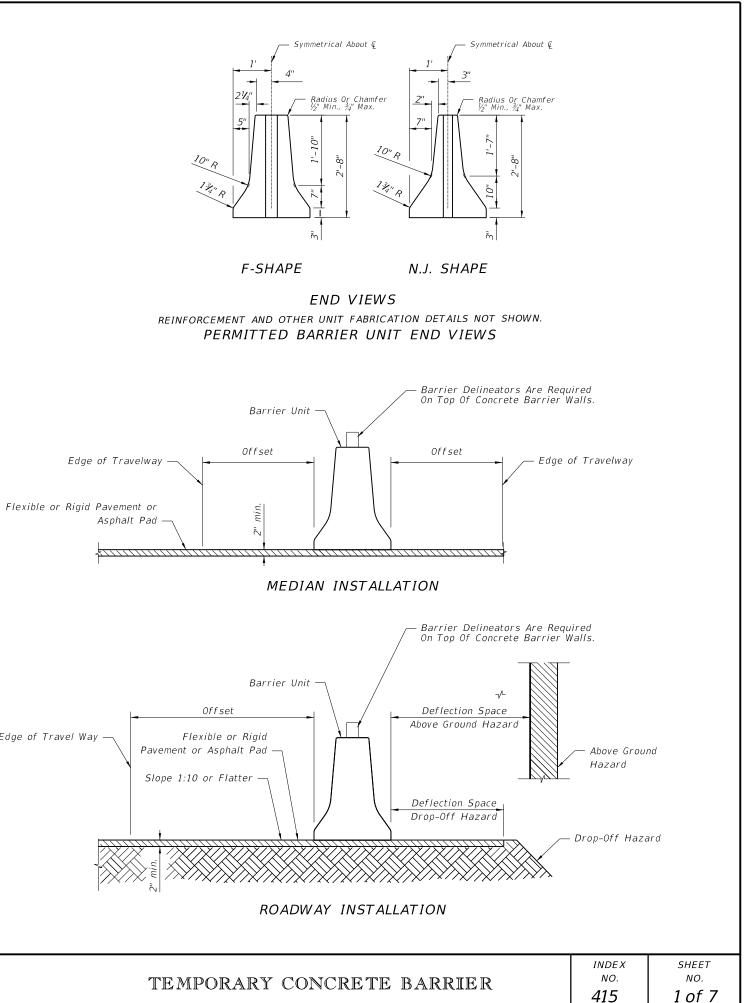
## GENERAL NOTES

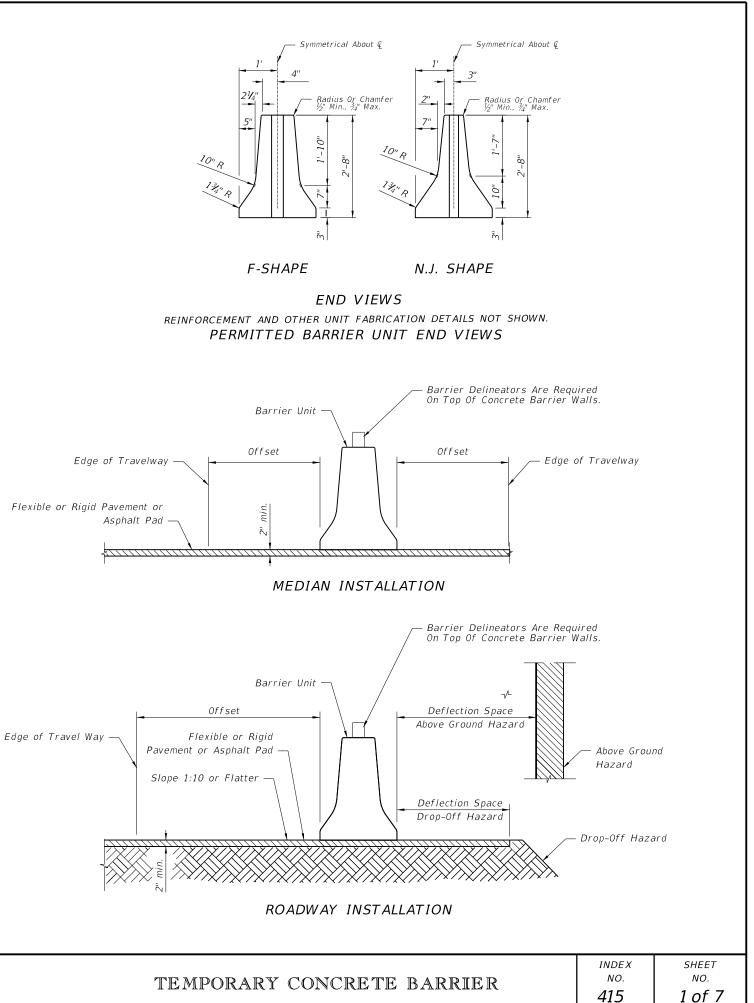
- 1. Temporary concrete barrier systems on roadways may be any of the following:
- a. The FDOT Type K Temporary Concrete Barrier system (Design Standard Index 414). F-Shape Units. For temporary concrete barrier systems on bridges see Design Standard Index No. 414.
- b. Proprietary temporary concrete barrier systems meeting NCHRP Report 350 Test Level 3 criteria which are included on the Qualified Products List.
- 2. Barrier units of dissimilar types may be interconnected within a single line barriers using transition units.
- 3. Alignment, length of need, anchorage and end treatment shall be in accordance with this Index.
- 4. Temporary concrete barrier units shown herein shall not be used for permanent barrier wall construction regardless of unit length.
- 5. If the plans specify Barrier (Temporary) (Type K), substitution with other barrier types is not permitted.
- 6. If the plans specify temporary concrete barrier system, substitution with water filled barriers is not permitted.
- 7. Where existing pavement is not present, construct an Asphalt Pad using Miscellaneous Asphalt Pavement. Cost of the Asphalt Pad to be included in the cost of the Barrier system.
- 8. Barrier Delineators are to be mounted on top of temporary concrete barriers that are used as barriers along traveled ways in work zones. The barrier delineators are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways.
- 9. Barrier units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier (Temporary), LF. Barrier delineators shall meet the requirements of Section 705 of the Standard Specifications for Road & Bridge Construction.
- 10. Deflection space shall be clear of any grass, construction debris, stockpiled materials, equipment, and objects.
- 11. Placing alternate temporary barrier systems with heights greater than 32 inches within the work zone may obstruct the clear sight distance at intersections and driveways. Prior to placing these barrier systems, the contractor shall submit a Certification Statement that the clear sight distance meets the requirements of Index 546, signed and sealed by a Florida Professional Engineer.
- 12. Minimum temporary concrete barriers installed per run shall be 16 units.

OFFSET AND DEFLECTION SPACE REQUIREMENTS				
Installation	Shielding	Work Zone Speed	Offset to Travelway	Deflection Space
	Above Ground	45 mph or Less	1' min, 2' preferred	2' min.
	Hazards	50 mph and Greater	2' min, 4' preferred	4' min.
Left or Right Shoulder		45 mph or Less	1' min, 2' preferred	2' min.
	Drop-Off	50 mph and Greater		
	Hazards	a. Drop-offs 4' or Less and NO traffic below	2' min, 4' preferred	2' min.
		b. All drop-off conditions other than 'a'	2' min, 4' preferred	4' min.
Separating	Adjacent Opposing	45 mph or Less	1' min, 2' preferred	1' min., 2' prefered
Traffic	Traffic	50 mph and Greater	2' min, 4' preferred	2' min., 4' preferred





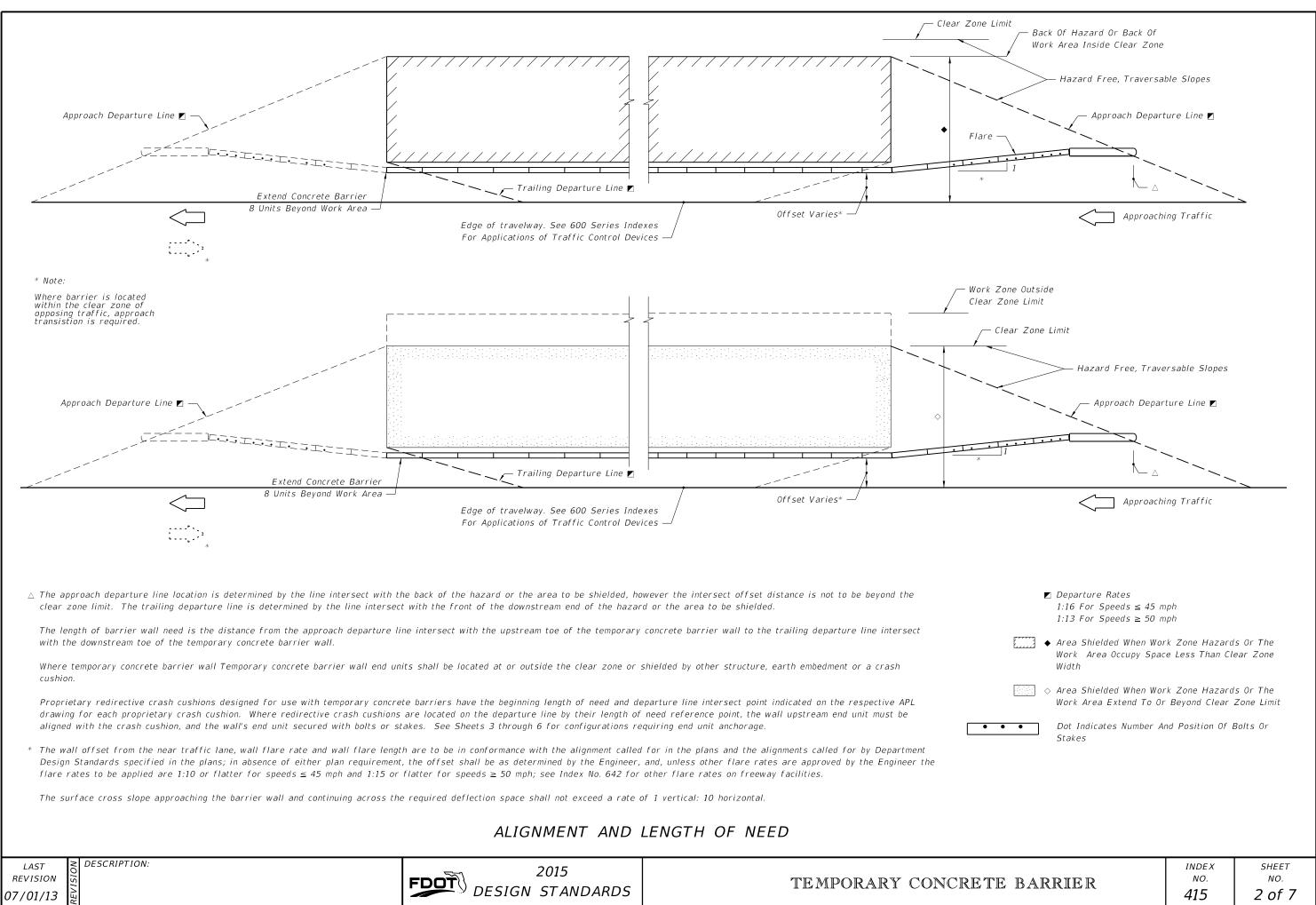


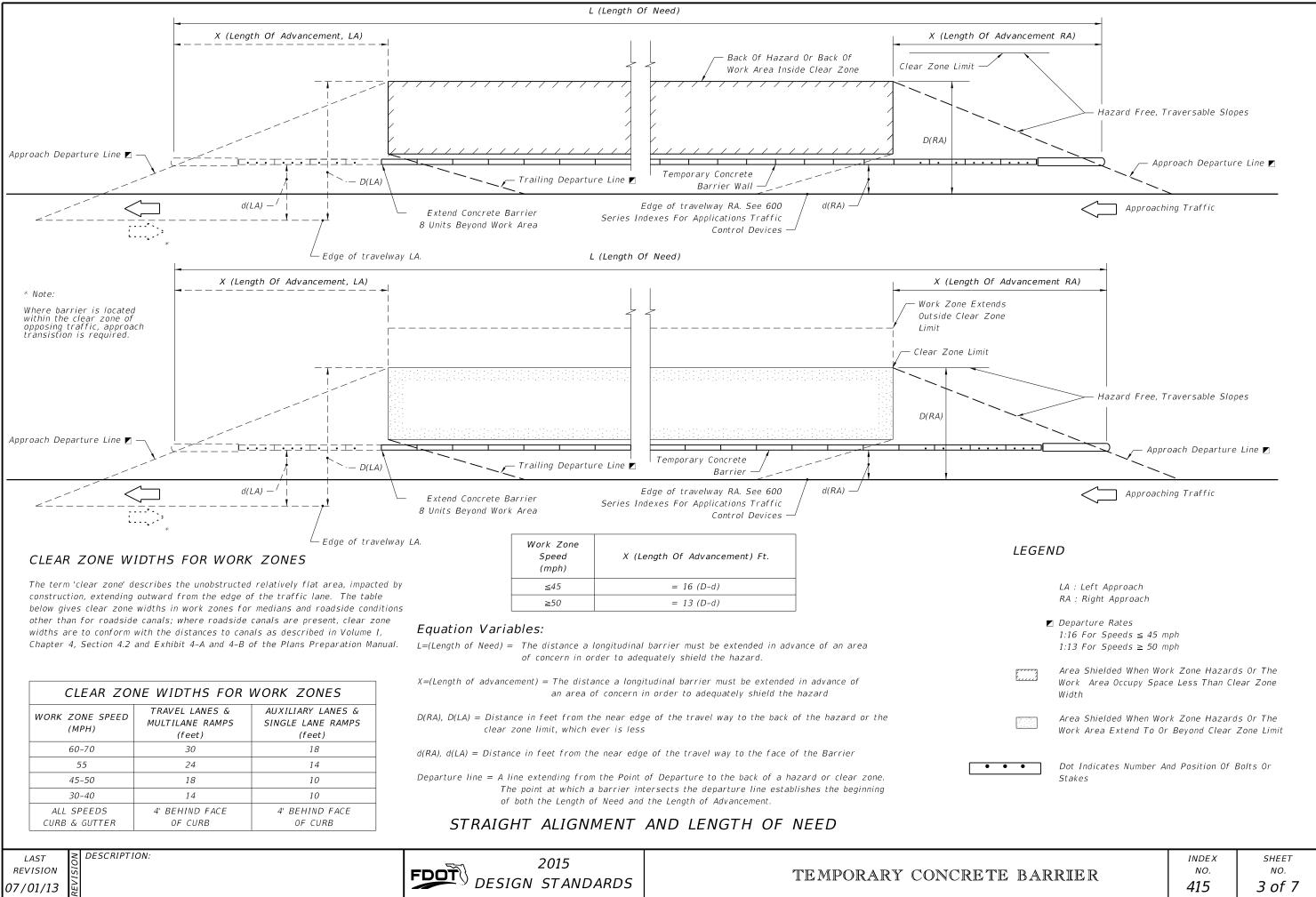


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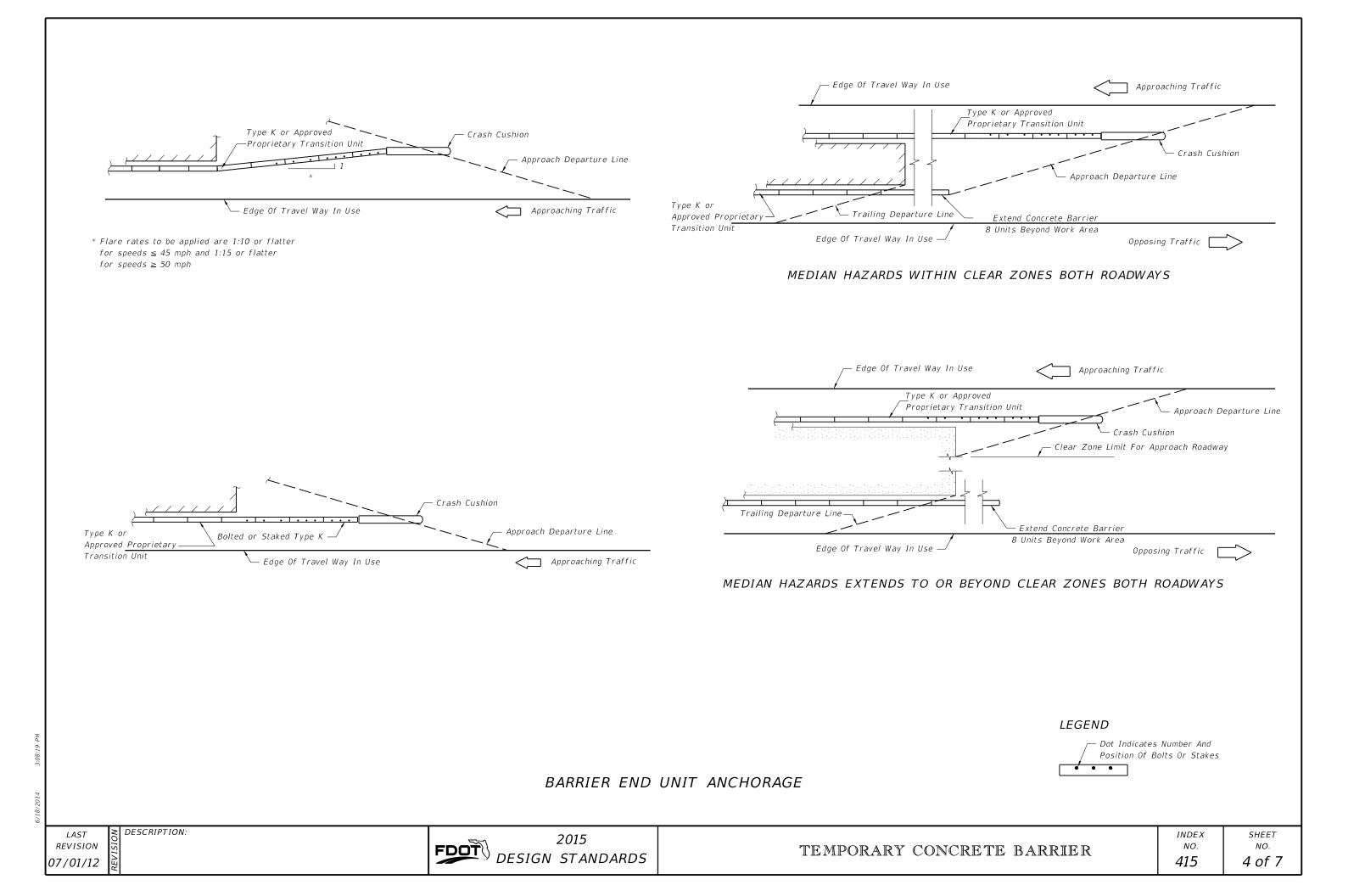


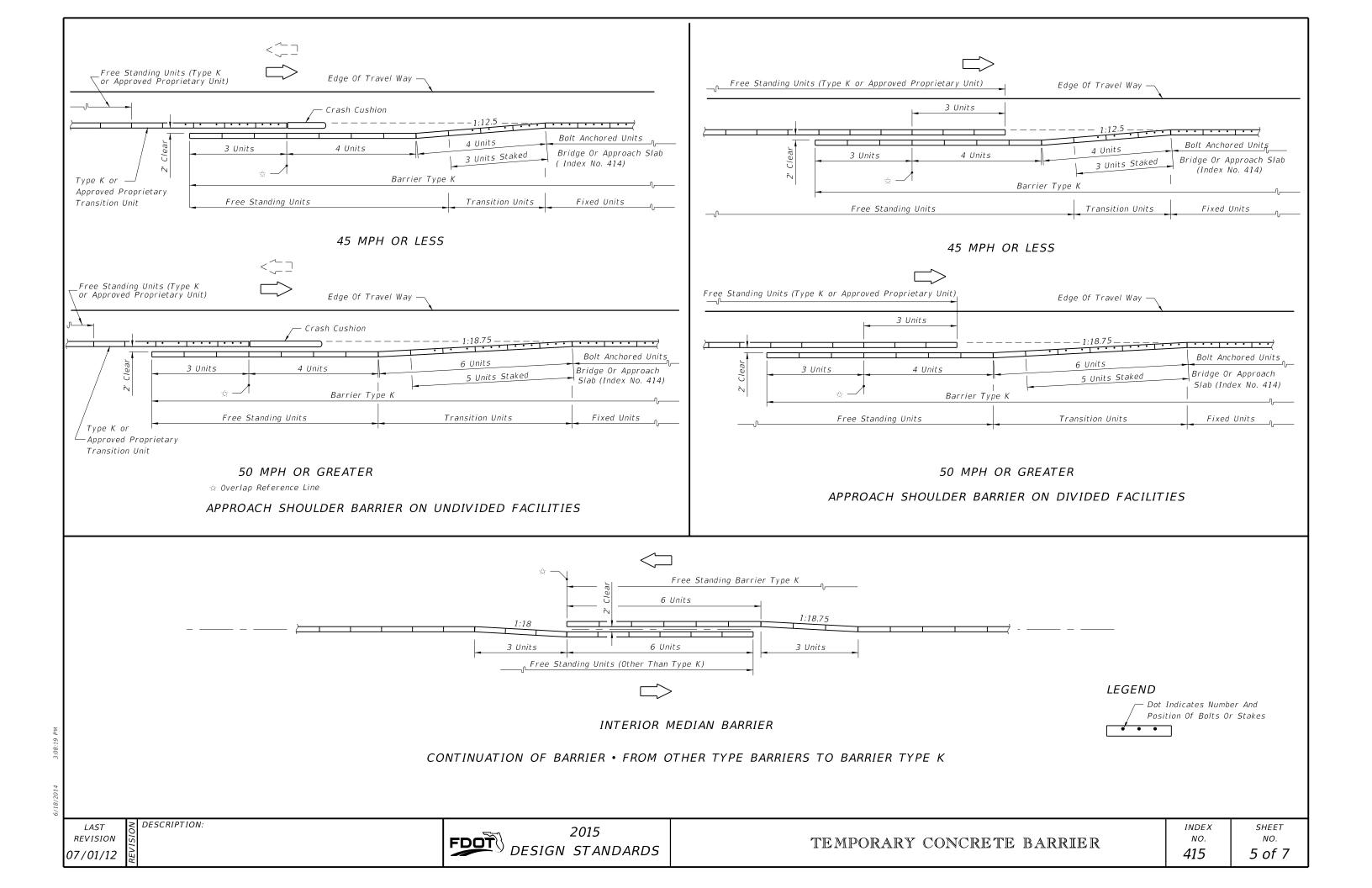


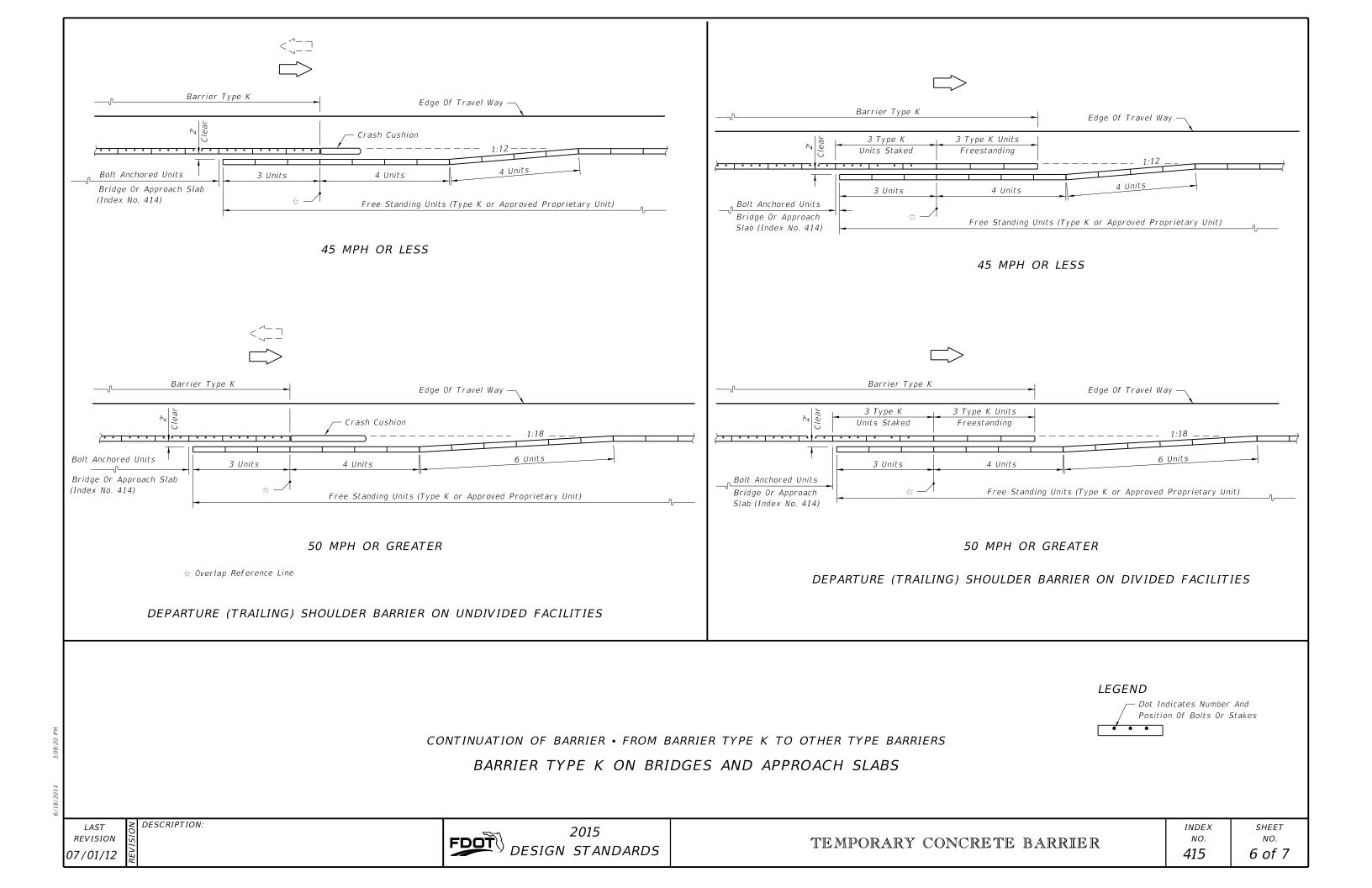


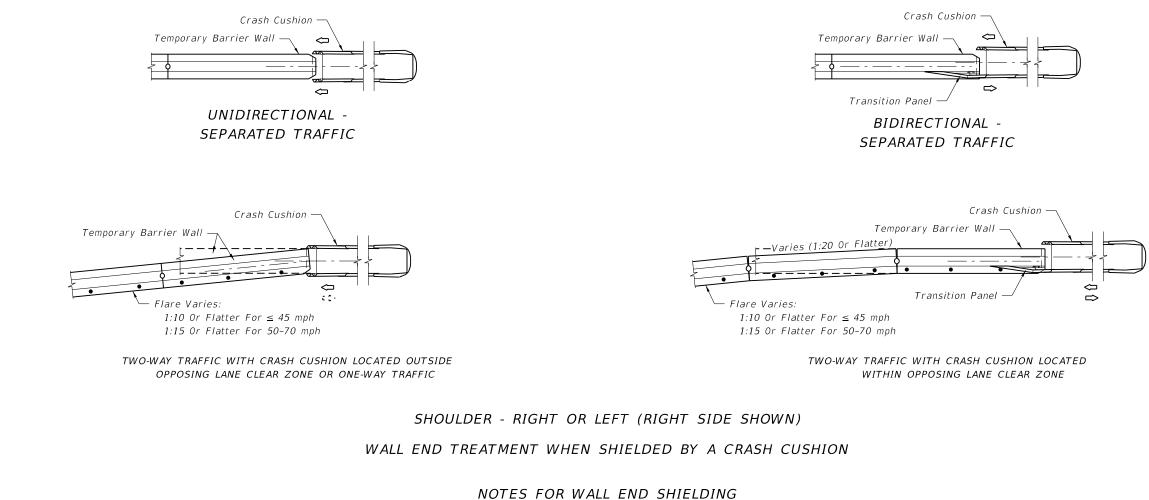
CLEAR ZONE WIDTHS FOR WORK ZONES			
WORK ZONE SPEED (MPH)	TRAVEL LANES & MULTILANE RAMPS (feet)	AUXILIARY LANES & SINGLE LANE RAMPS (feet)	
60-70	30	18	
55	24	14	
45-50	18	10	
30-40	14	10	
ALL SPEEDS CURB & GUTTER	4' BEHIND FACE OF CURB	4' BEHIND FACE OF CURB	











- 1. Redirective crash cushions are the principal (standard) device to be used for shielding approach ends of temporary concrete barrier walls. The contractor has the option to construct any of the redirective crash cushions listed on the Qualified Products List at "102 Temporary Crash Cushion", subject to the uses and limitations described on their respective drawings. The barrier wall four end unit abutting crash cushions must be anchored to a paved surface using anchors/stakes in accordance with Standard Index 414.
- 2. Temporary redirective crash cushions shall be installed in accordance with the manufacturer's specifications and recommendations. Temporary crash cushions can be either new or functionally sound used devices. Performance of intended function is the only condition for acceptance, whether the crash cushion is new, used, refurbished, purchased, leased, rented, on loan, shared between projects, or made up of mixed new and used components.
- 3. Temporary Crash Cushions shall not be bolted down on bridge superstructures that contain post-tensioned tendons within the concrete deck (top flange of concrete box girders) or on bridge superstructures consisting of longitudinally prestressed, transversely post-tensioned, solid or voided concrete slab units. Gating crash cushions shall be used where bolting is not allowed.
- 4. Assemble and install Crash Cushions according to the limitations noted on the Approved Products List (APL) webpage, the manufacturer's specifications, and the applicable crash cushion drawings posted on the APL.

### 5. Optional temporary redirective crash cushions are to be paid for per locations under the contract unit price for Crash Cushion (Redirective Option) (Temporary), LO.

6. A yellow Type I Object Marker shall be centered 3' in front of the crash cushion nose. Mounting hardware shall be in conformance with Section 993 of the Standard Specifications for Road and Bridge Construction.

As an option, the contractor may install reflective sheeting on the nose of the crash cushion. The sheeting to be used must be solid yellow, Type III or better and must be a product listed on the Department's Approved Products List (APL). The sheeting to be applied to the nose of the crash cushion shall be a minimum of 360 square inches with a minimum height of 15 inches.

- 7. Equipment, stockpile material, etc., shall not be placed behind the crash cushion.
- 8. When subjected to reverse direction hits, construct Transition Panels from Concrete Barrier Walls to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.
- 9. Galvanize metallic components to meet the requirements for Steel Guardrail, Section 967 of the Standard Specifications for Road and Bridge Construction.

LEGEND

Dot Indicates Number And Position Of Bolts Or Stakes • •

DESCRIPTION:

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LAST REVISION

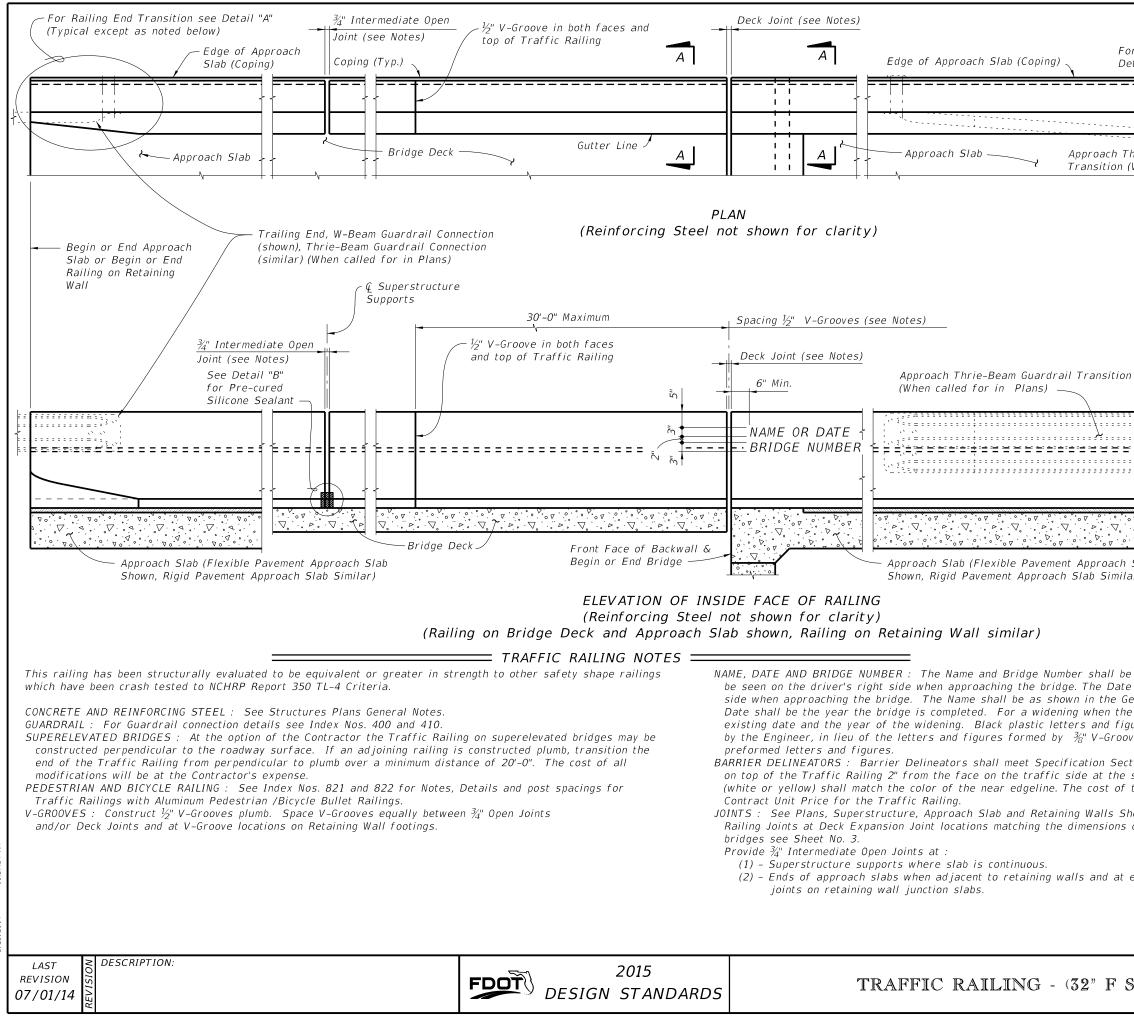
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SHIELDING WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTIO

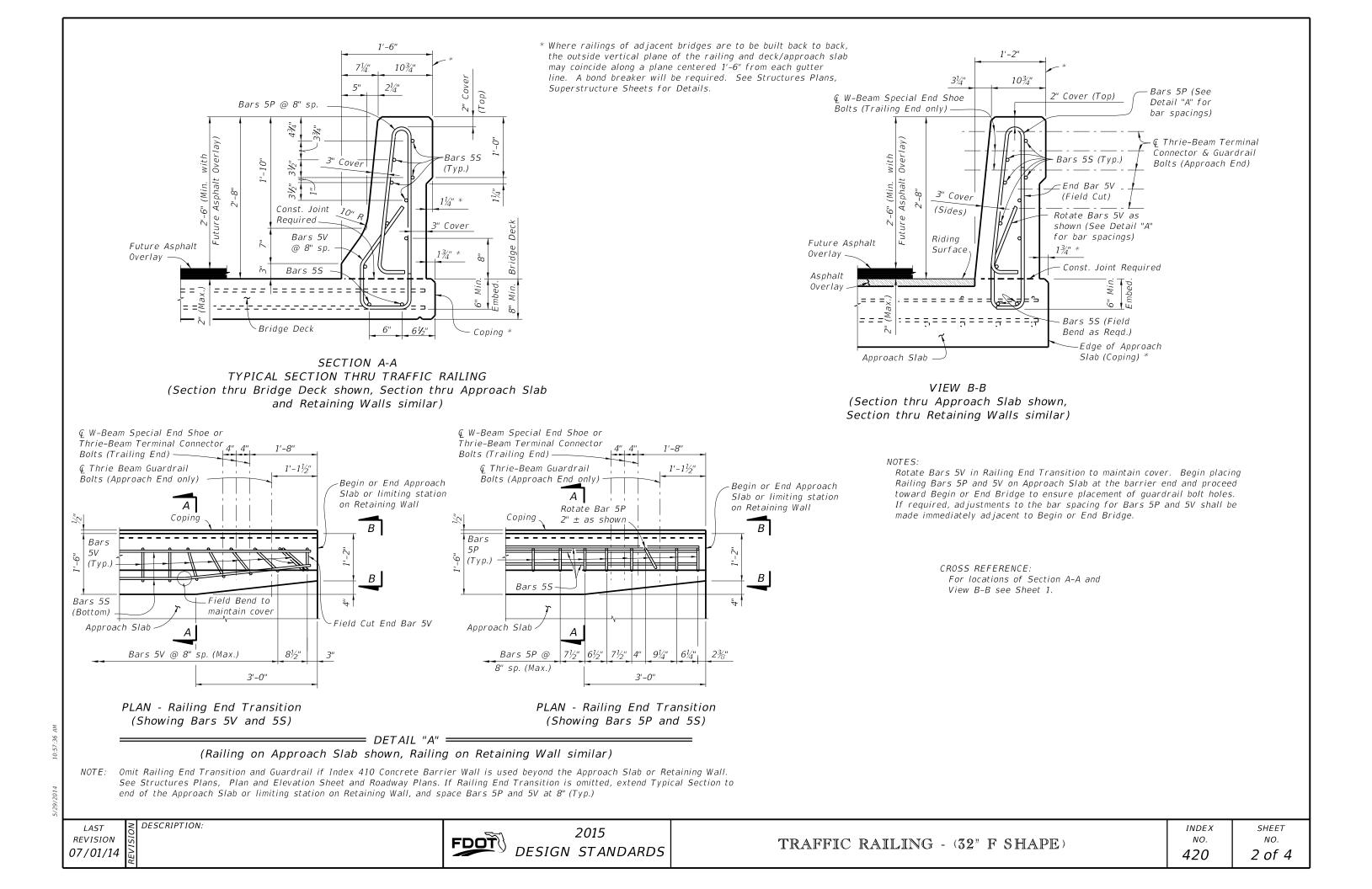
2015 DESIGN STANDARDS	TEMPO
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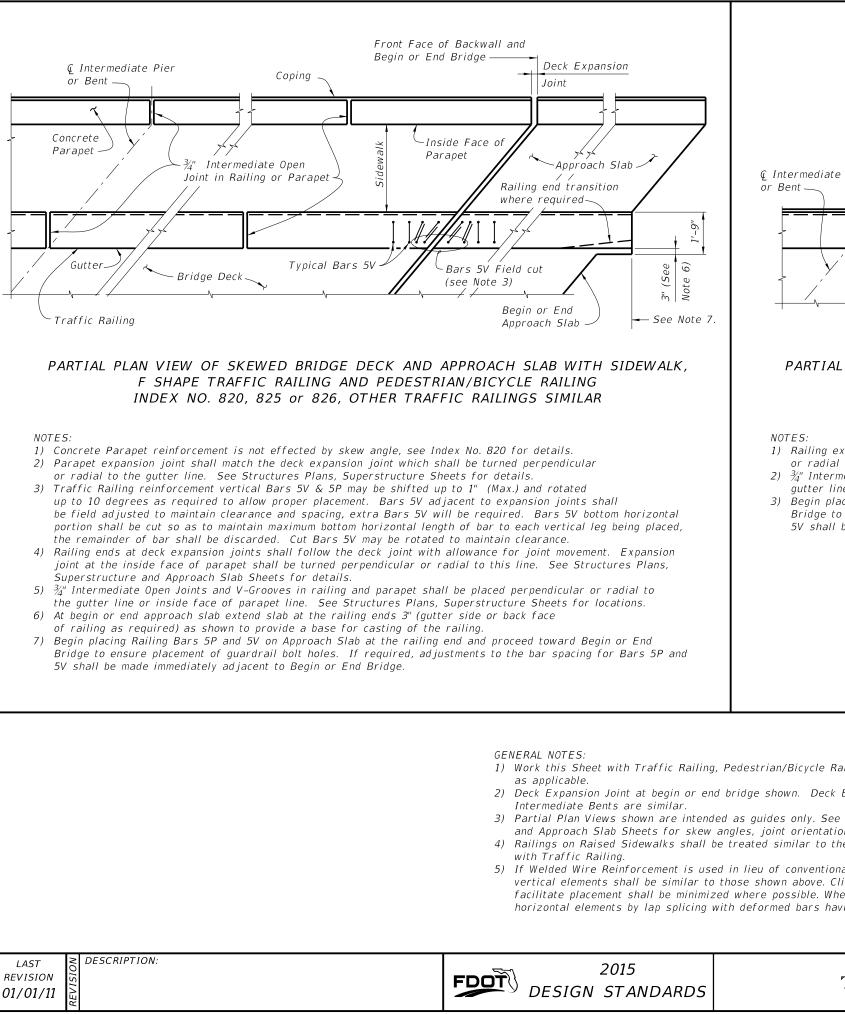
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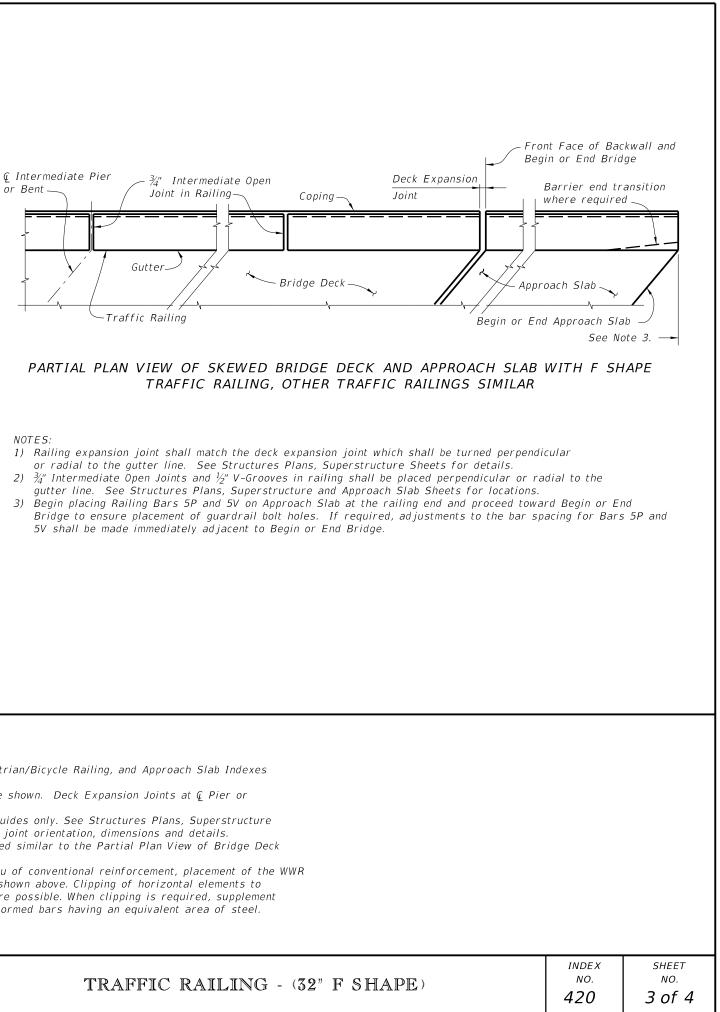
E BARRIER	INDEX	SHEET
	NО. <b>Д15</b>	NO. 7 of 7
	715	/ 0/ /



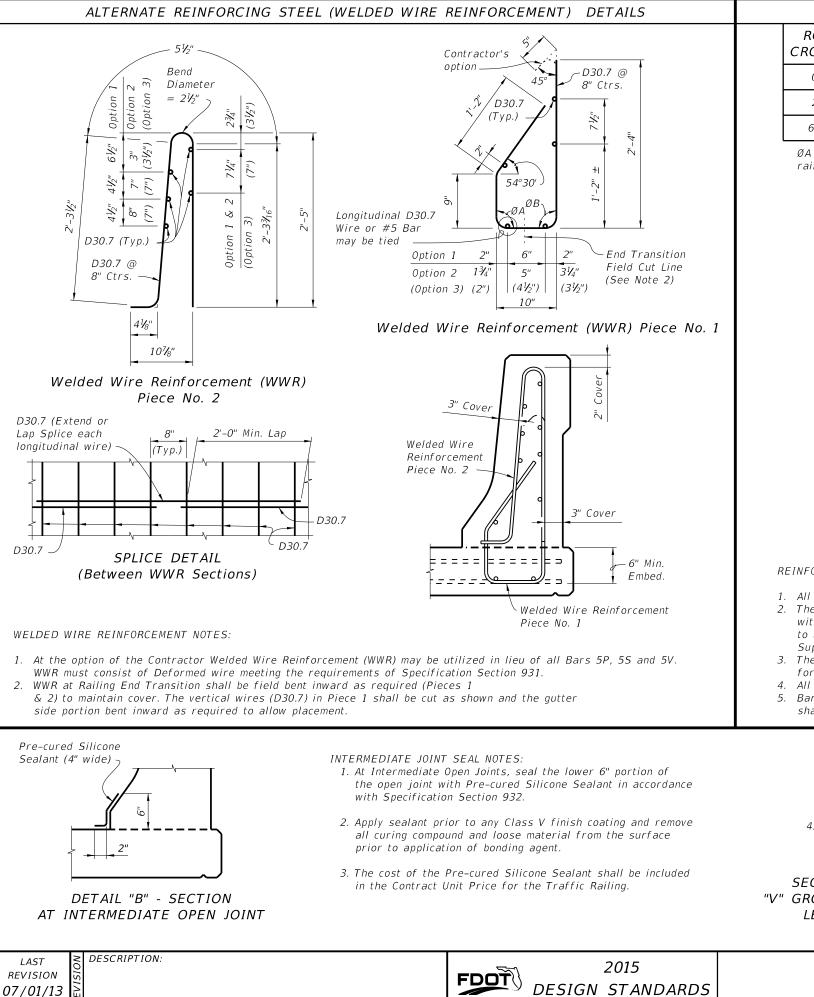
r Railing	End Transition see		
	ypical except as noted)		
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		/	
· · · · ·		(	· )
	· • • • • • • • • • • • • • • • • • • •		
	Guardrail		В
when can	ed for in Plans)		
	Begin or End Approach	Slab or Begi	n
	or End Railing on Reta		
			<b></b> _
	BARRIER DEL		
	SPACIN	VG	
	Distance –		
	Edge of Travel Lane	Spacing (Ft	.)
	to Face of Railing		<b></b>
	< 4'	40'	
	4' to 8'	80'	
	> than 8'	None Requir	ed
		none negan	
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Slab ar, Typ.) –			
,, p.,,	CROSS REF	ERENCE:	
		on A-A, View E	B-B and
		, see Sheet 2.	
	For Detai	l "B", see She	et 4.
nlaced or	n the Traffic Railing so	as to	
shall be	n the Traffic Railing so placed on the driver's le	eft	
eneral Not	es in the Structures Pla	ns. The	
	railing is removed, use		
ures 3" in height may be used, as approved /es. V-Grooves shall be formed by			
tion 993. Install Barrier Delineators			
spacing shown in the table above. Barrier Delineator color the Barrier Delineators shall be included in the			
the barrier benneators shar be included in the			
neets for actual dimensions and joint orientation. Provide open			
of the Deck Joint. For treatment of Railings on skewed			
expansion			
		INDEX	SHEET
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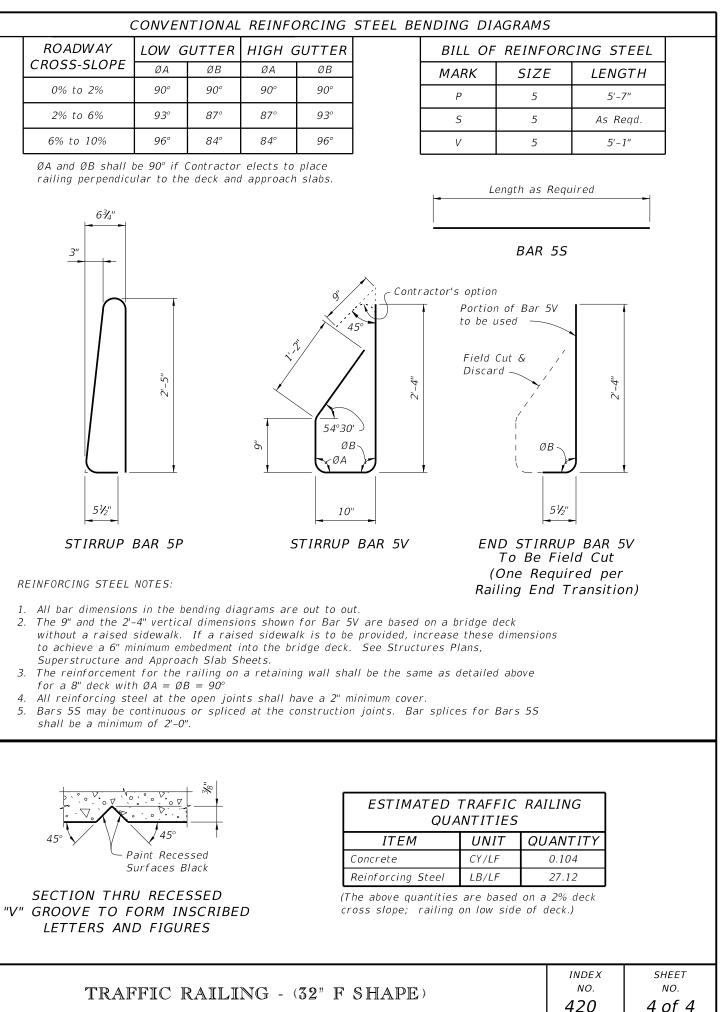


- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at Q Pier or
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.



	CONVENTIONAL REINFORCING S					
ROADWAY		LOW GUTTER		HIGH GUTTER		
	CROSS-SLOPE	ØA	ØB	ØA	ØB	
	0% to 2%	90°	90°	90°	90°	
	2% to 6%	9 <i>3°</i>	87°	87°	93°	
	6% to 10%	96°	84°	84°	96°	]

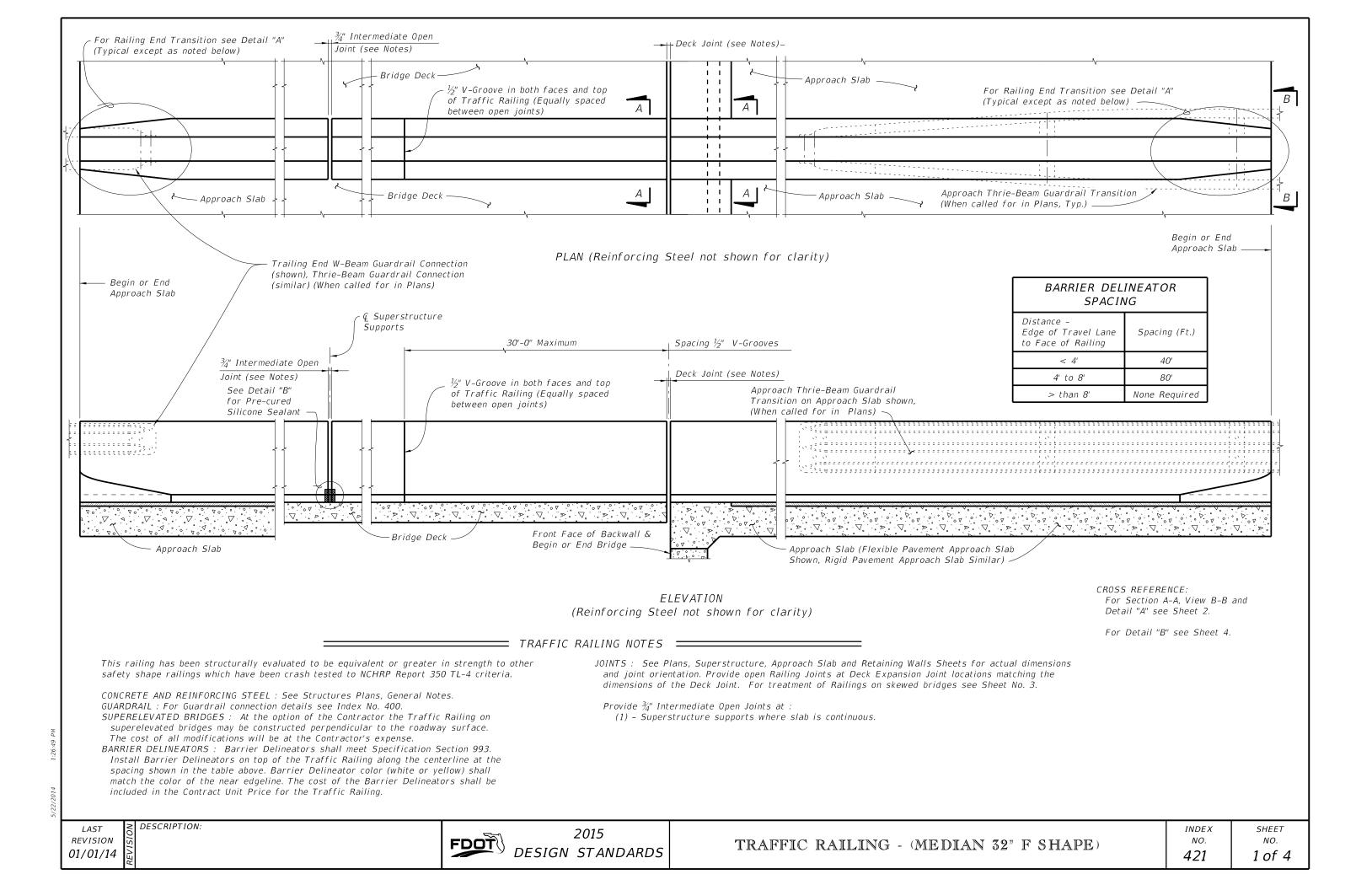
railing perpendicular to the deck and approach slabs.

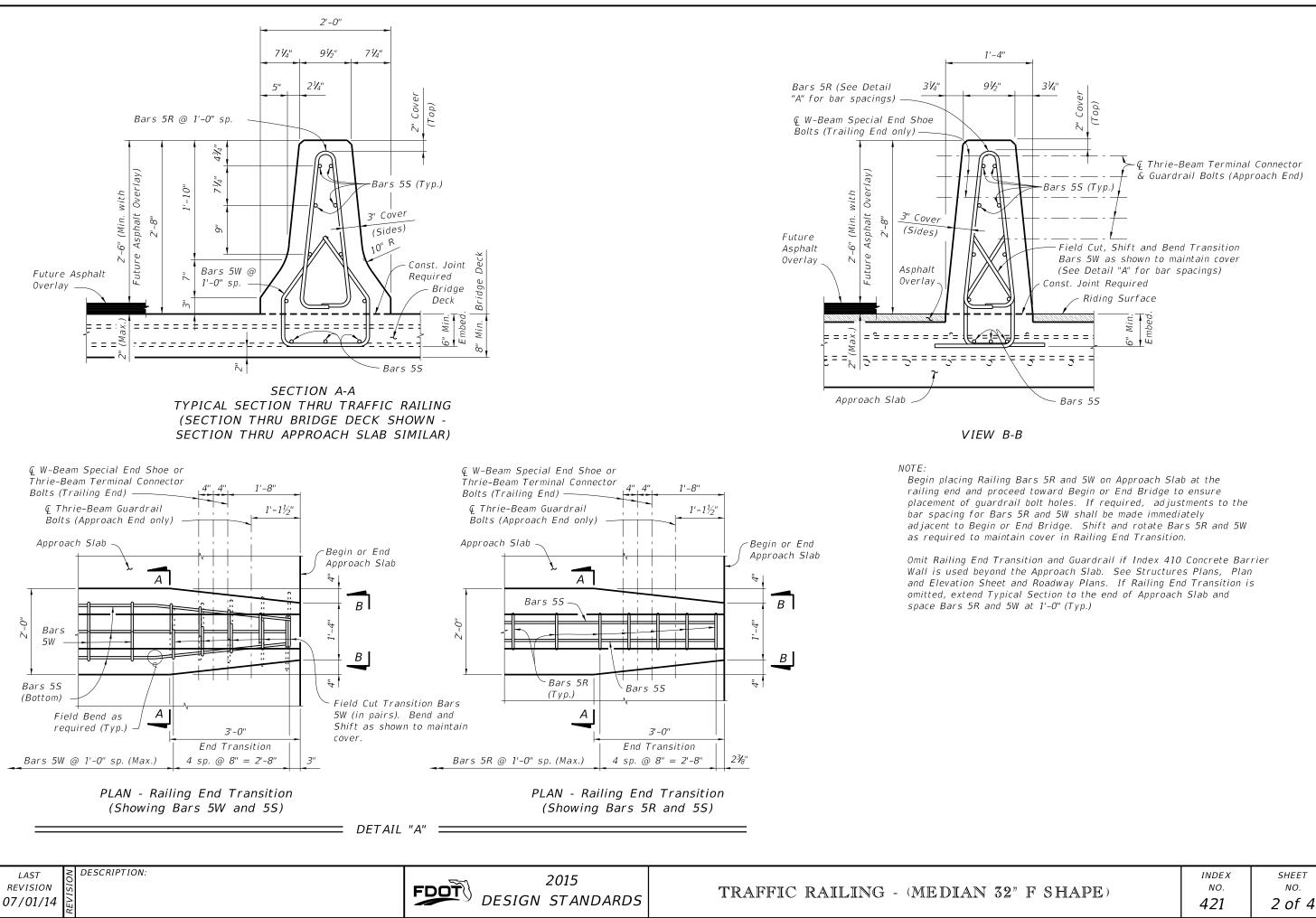


REINFORCING STEEL NOTES:

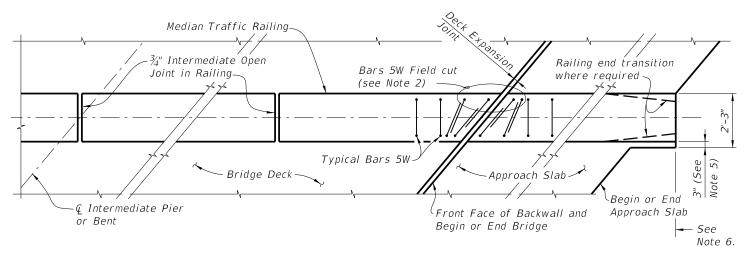
- 1. All bar dimensions in the bending diagrams are out to out.
- Superstructure and Approach Slab Sheets.
- for a 8" deck with  $\emptyset A = \emptyset B = 90^{\circ}$
- shall be a minimum of 2'-0".







	INDEX	SHEET
" F SHAPE)	NO.	NO.
	421	2 of 4



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

### NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4)  $\frac{3}{4}''$  Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the Q of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at *Q* Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing deformed bars with an equivalent area of steel.

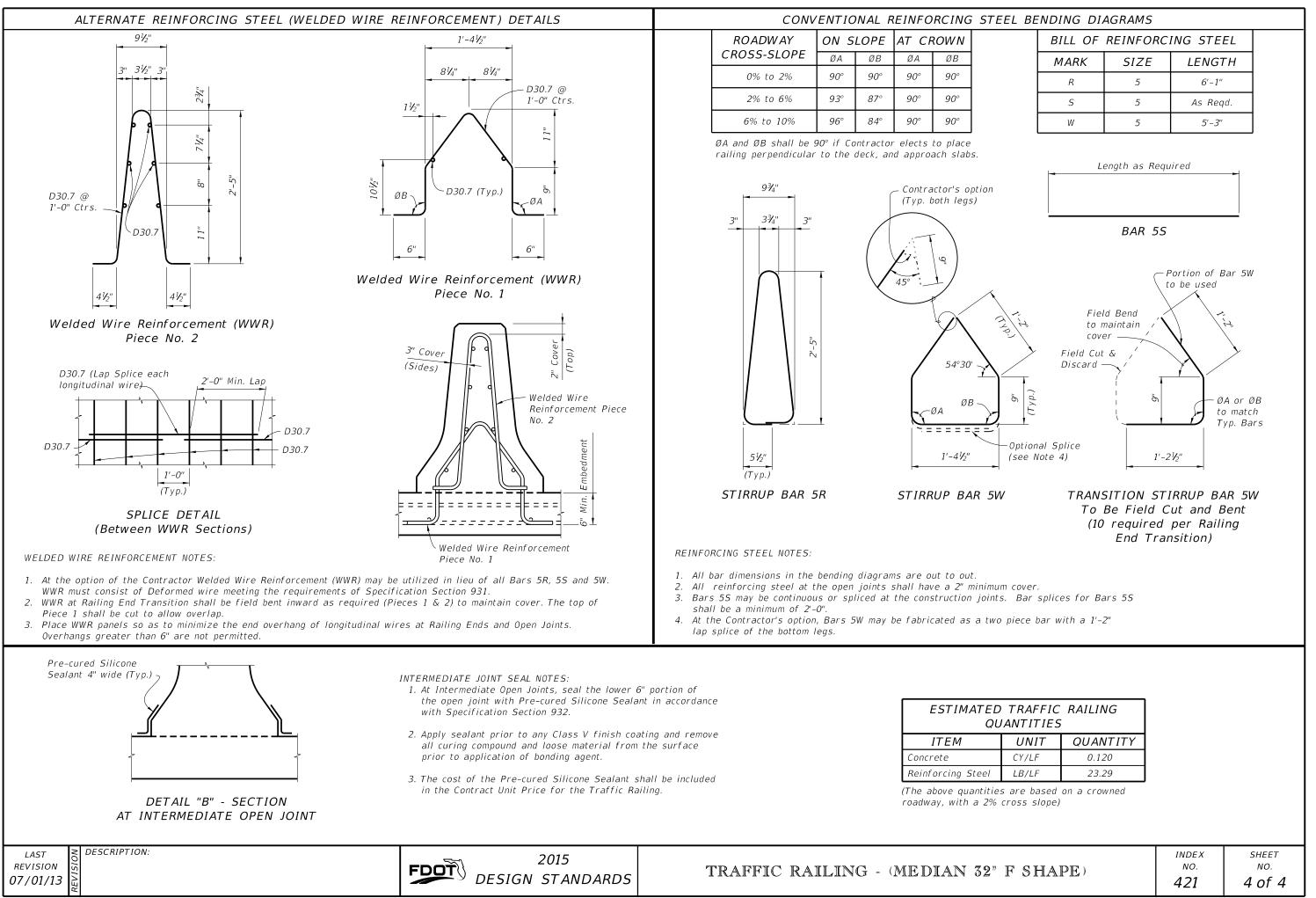
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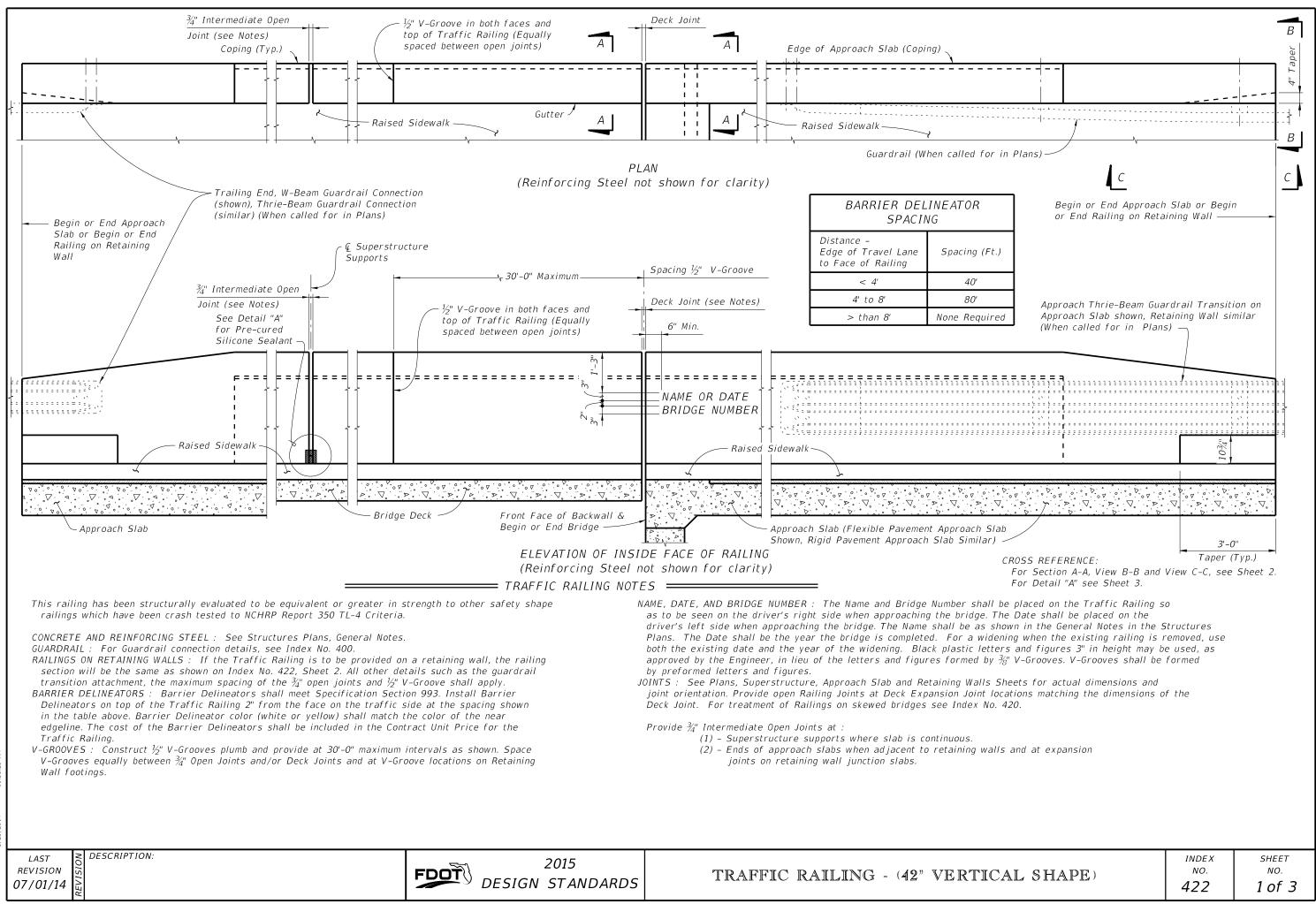
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	DESIGN STANDARD	S	

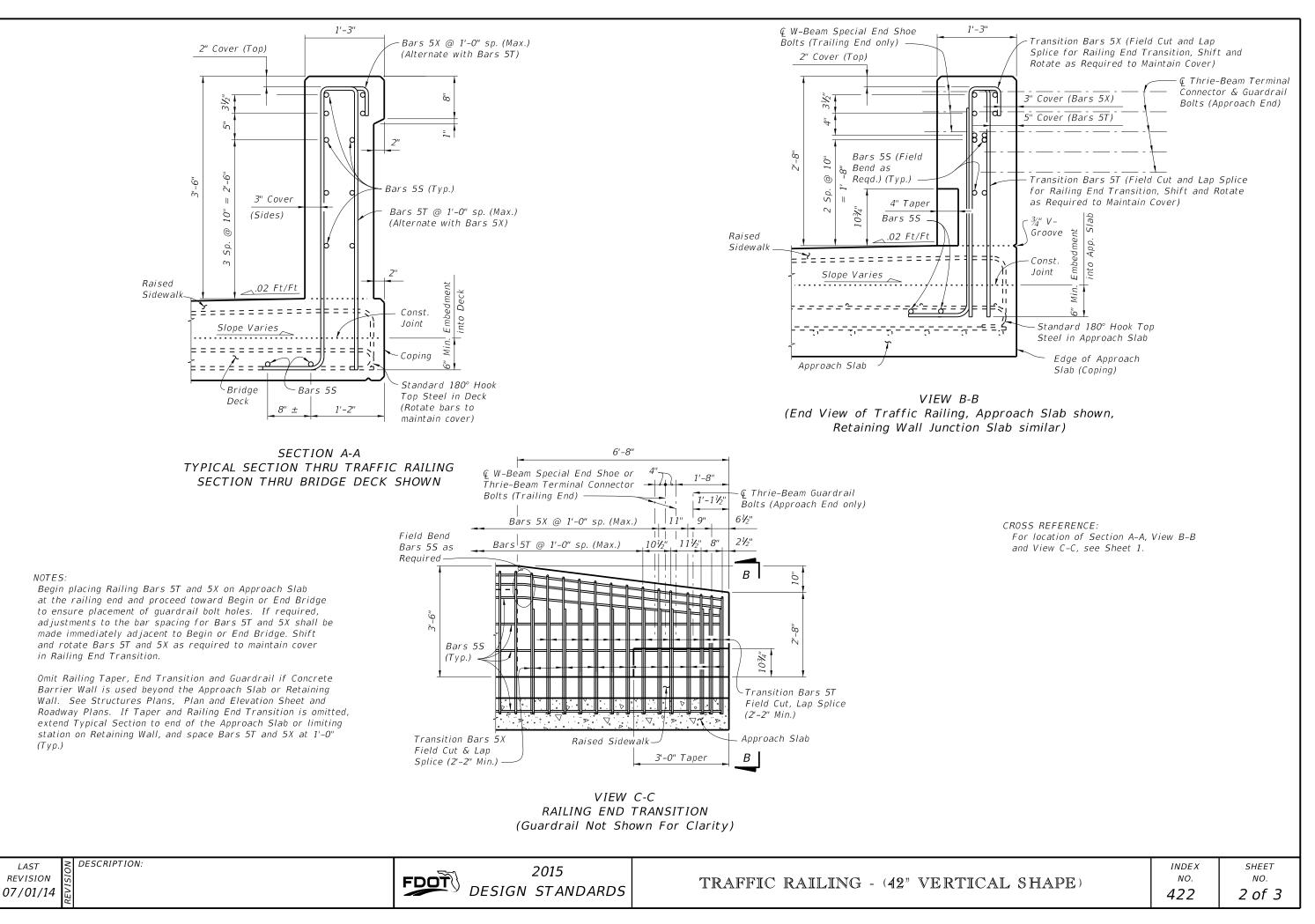
TRAFFIC RAILING - (MEDIAN 32

INDEX NO.	SHEET NO.

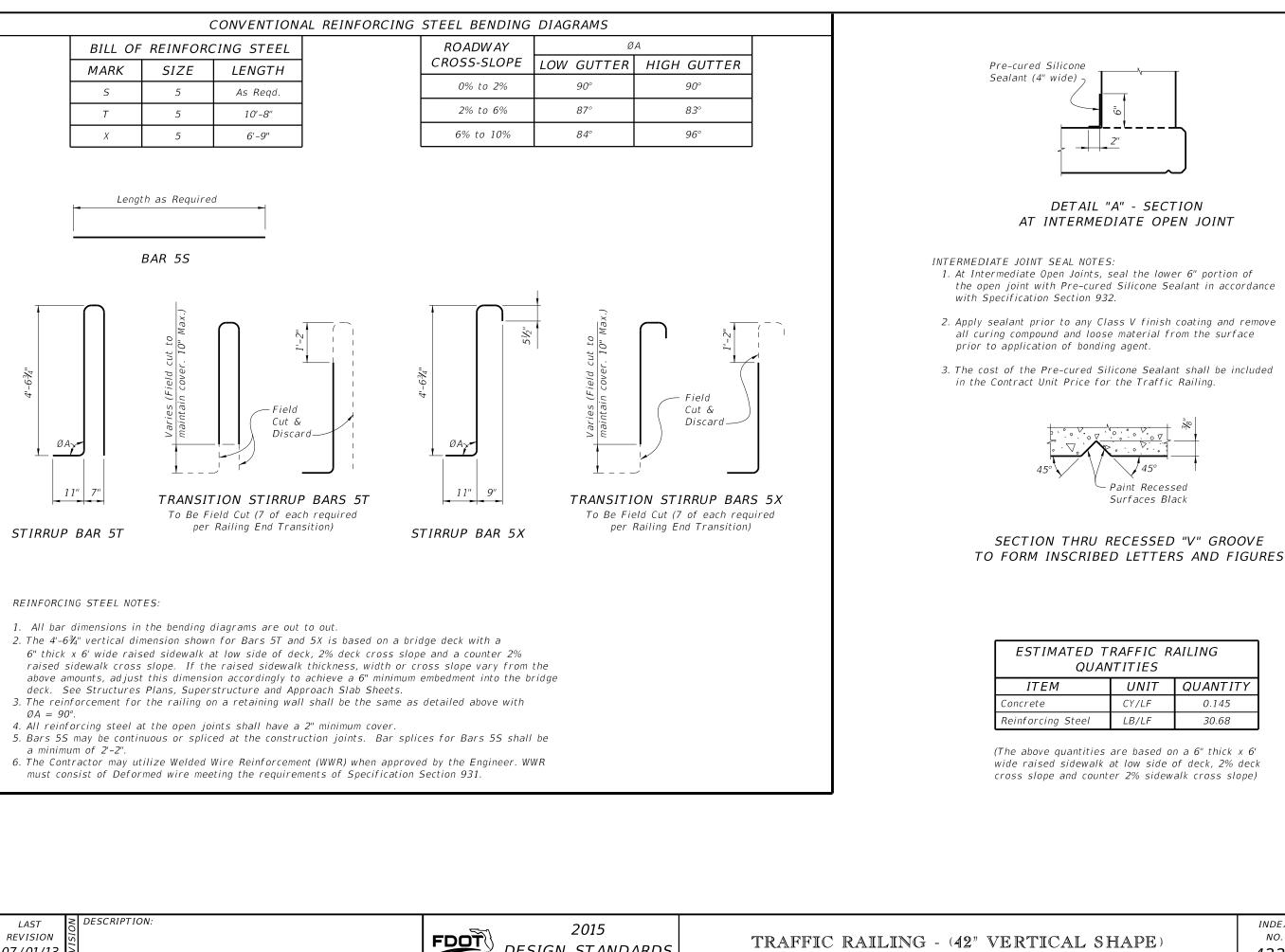


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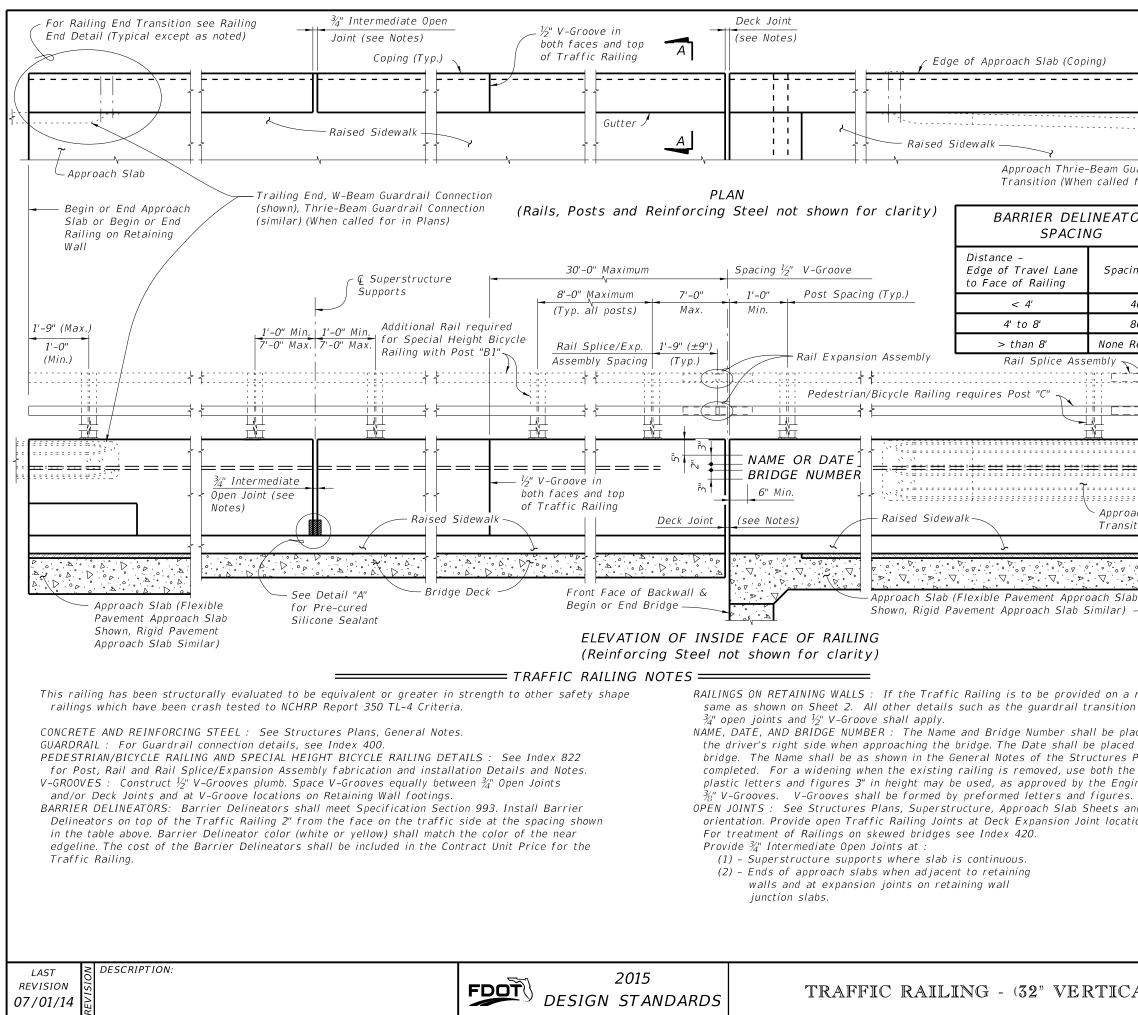
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DESIGN STANDARDS

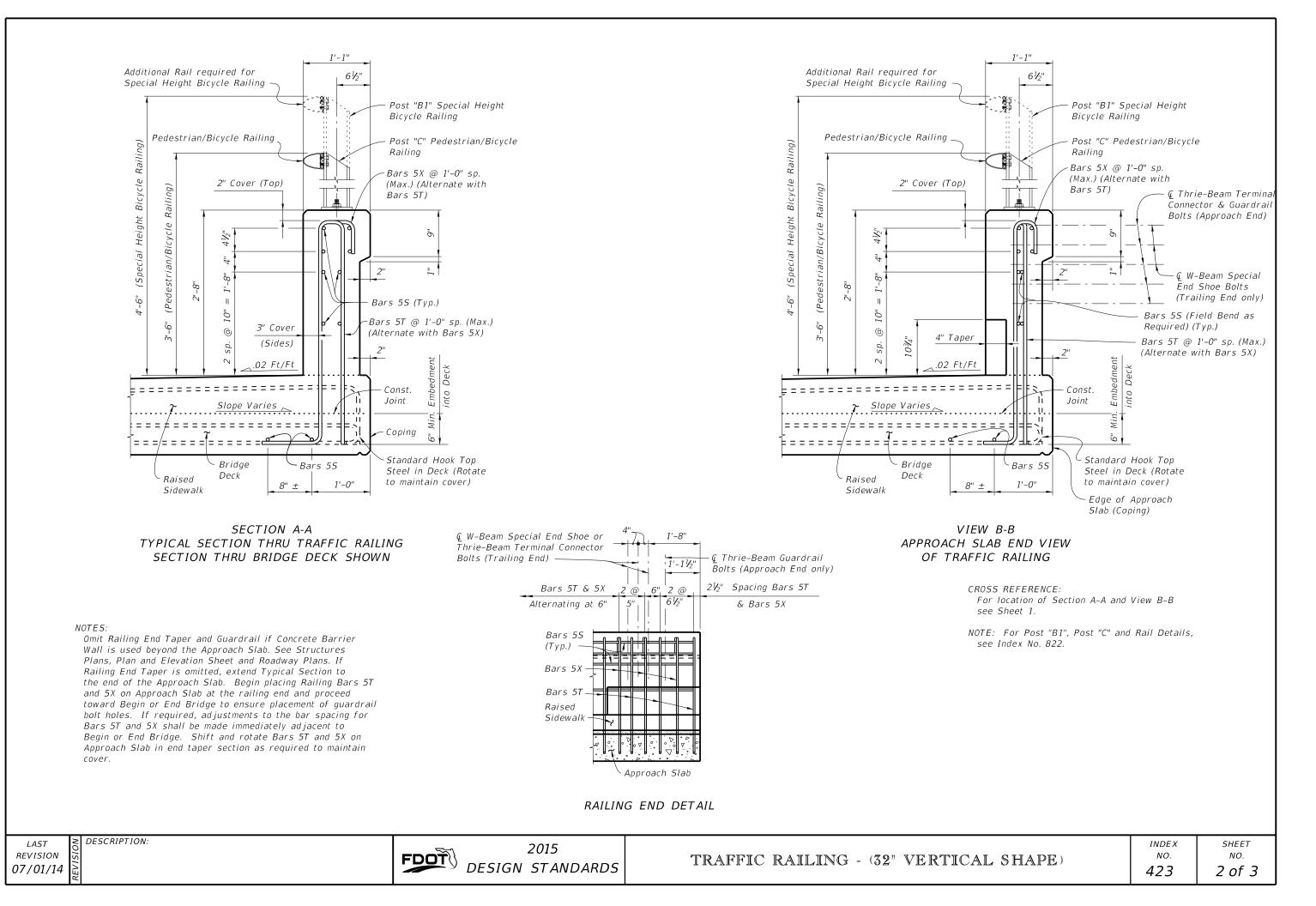
TRAFFIC RAILING - (42" VERTIC

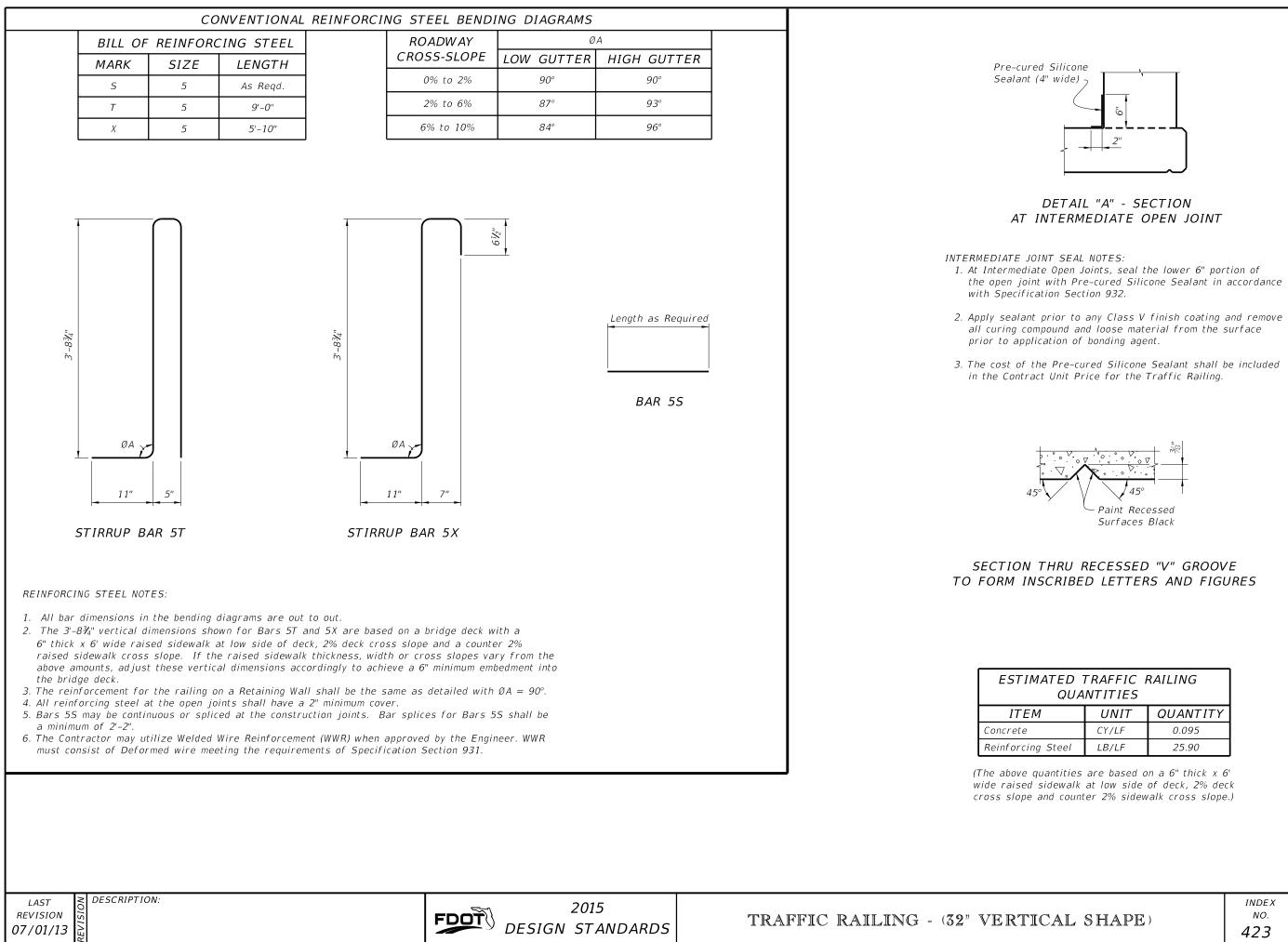
ATED TRAFFIC RAILING QUANTITIES				
	UNIT	QUANTITY		
	CY/LF	0.145		
Steel	LB/LF	30.68		

	INDEX	SHEET
AL SHAPE)	NO.	NO.
AL SHAPE)	422	3 of 3



For Railing End Transition s (Typical except as noted) —	see Railing End	l Detail <b>B</b>
(Typical except as noted) —		aper
		+
······		
uardrail		B
for in Plans)		
	End Approach or End Railin g Wall	
ing (Ft.) When Railing is provide Tapered at terminus of L	End Transitic	n
80' Tapered End Tr	ansition <sup>1</sup>	-9" (Max.)
Required		1'-0" (Min.)
Max.	 	
	,	(
	`	
		<u></u>
ach Thrie-Beam Guardrail ition (When called for in Plans)	103/	
$\neg \nabla, \neg \nabla,$		$\nabla$ ,
b	Begin or Approach	
	3'-0 Tape	
retaining wall, the railing section n attachment, the maximum spacin		
aced on the Traffic Railing so as d on the driver's left side when a Plans. The Date shall be the yea e existing date and the year of th ineer, in lieu of the letters and f	pproaching the ar the bridge i he widening. E	e s Black
nd Retaining Walls for actual dim ions matching the dimensions of u		
AL SHAPE)	INDEX NO. <b>423</b>	sнеет NO. <b>1 of 3</b>
	,23	107.5

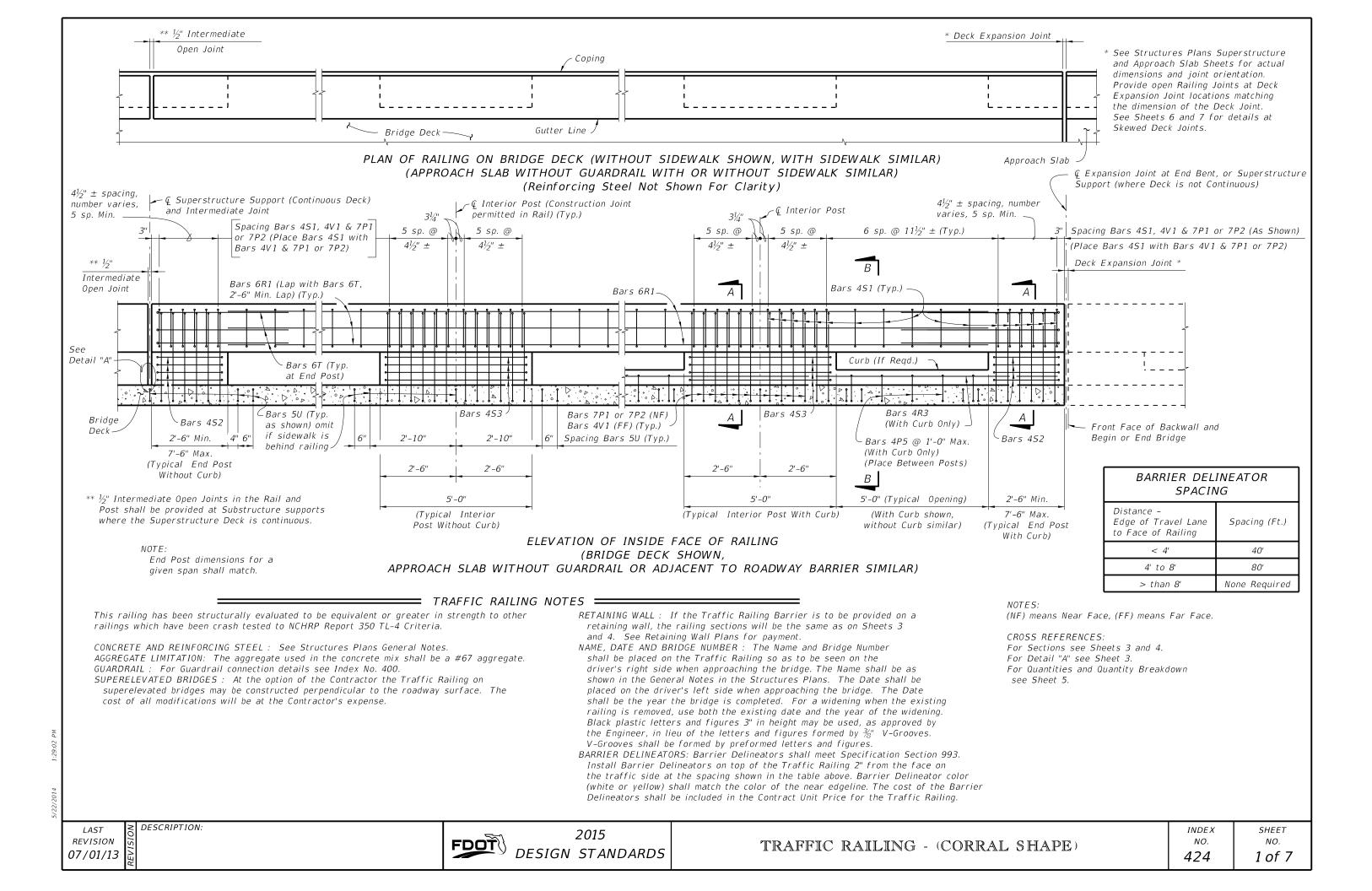


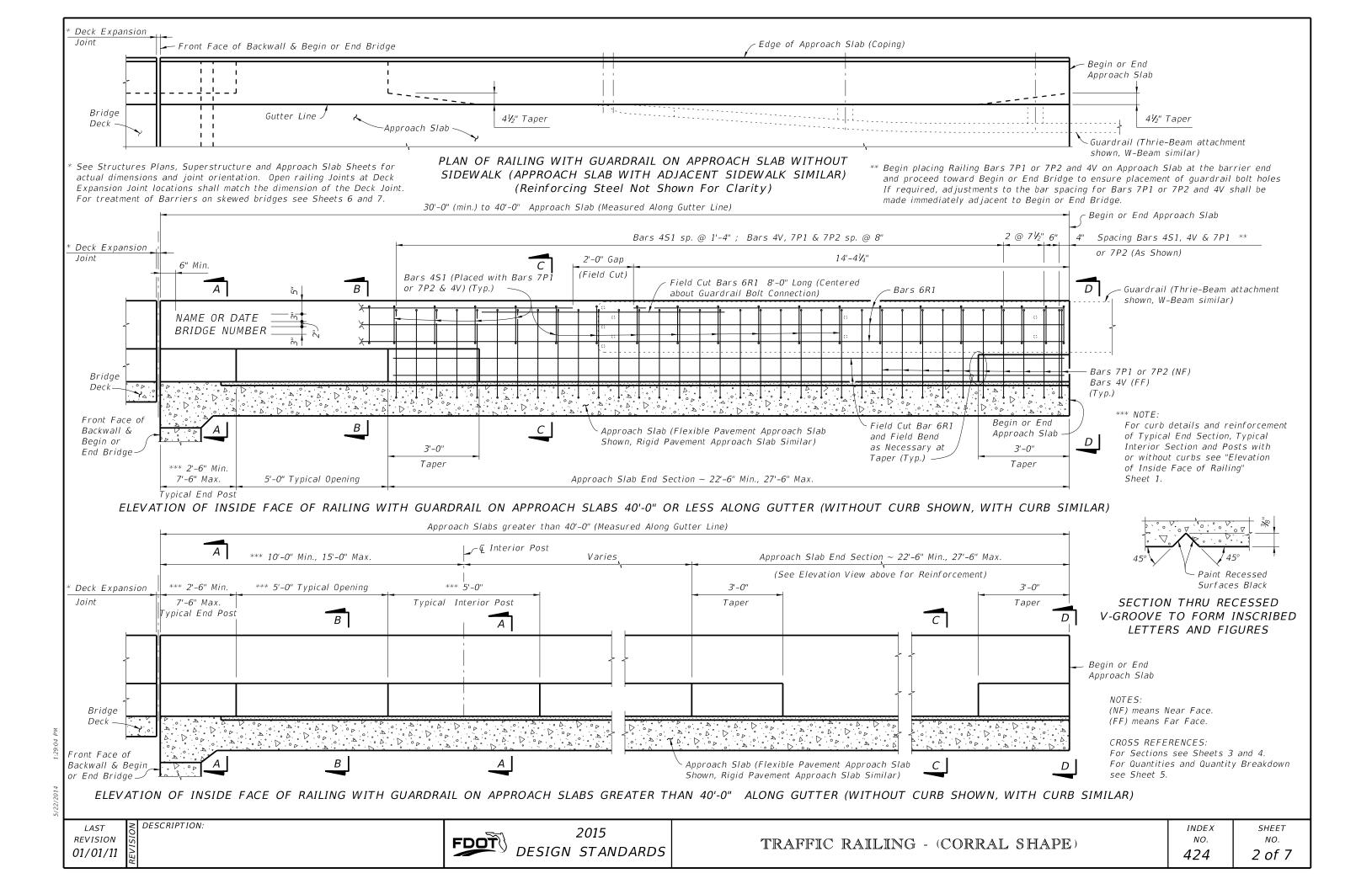


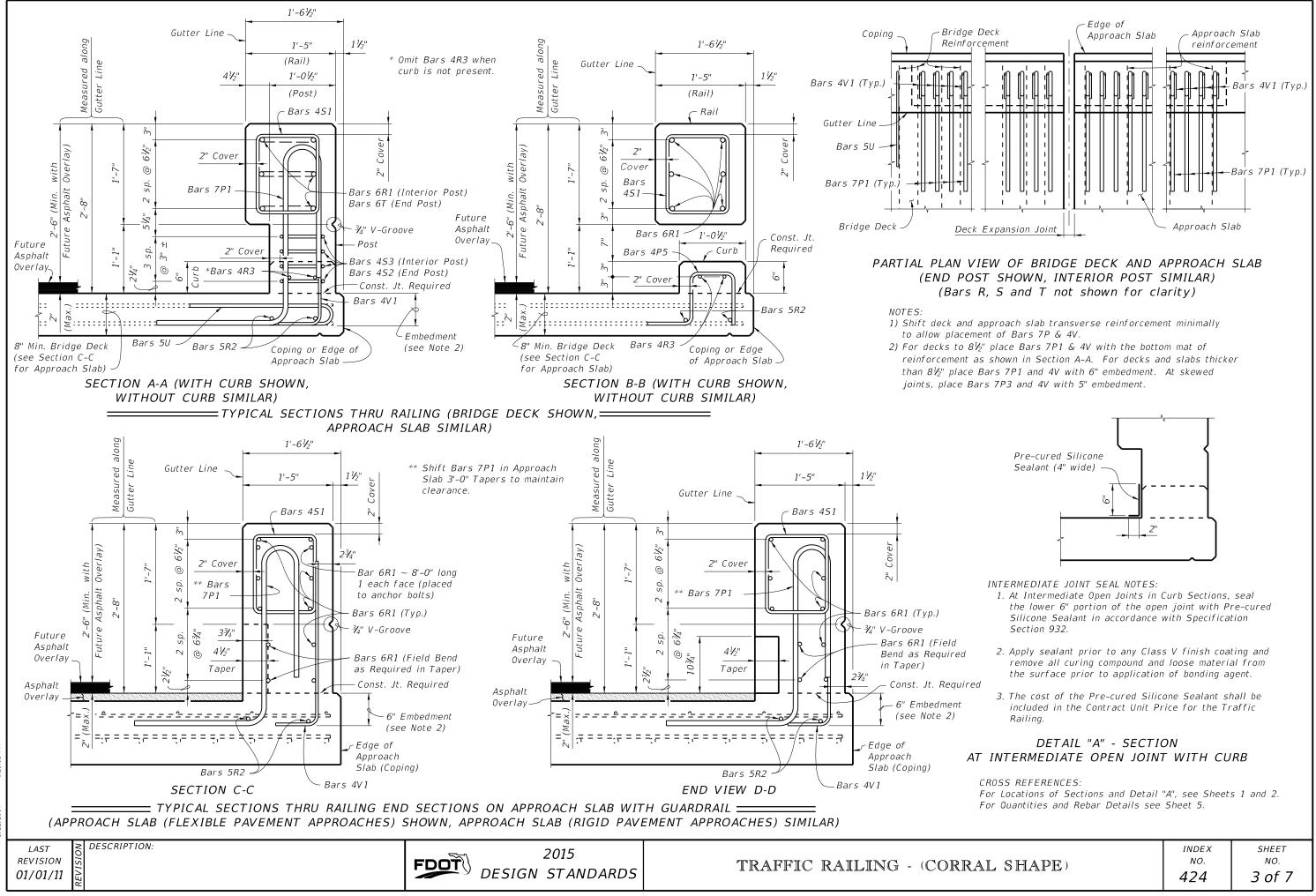
the open joint with Pre-cured Silicone Sealant in accordance

	TRAFFIC NTITIES	RAILING
	UNIT	QUANTITY
	CY/LF	0.095
Steel	LB/LF	25.90

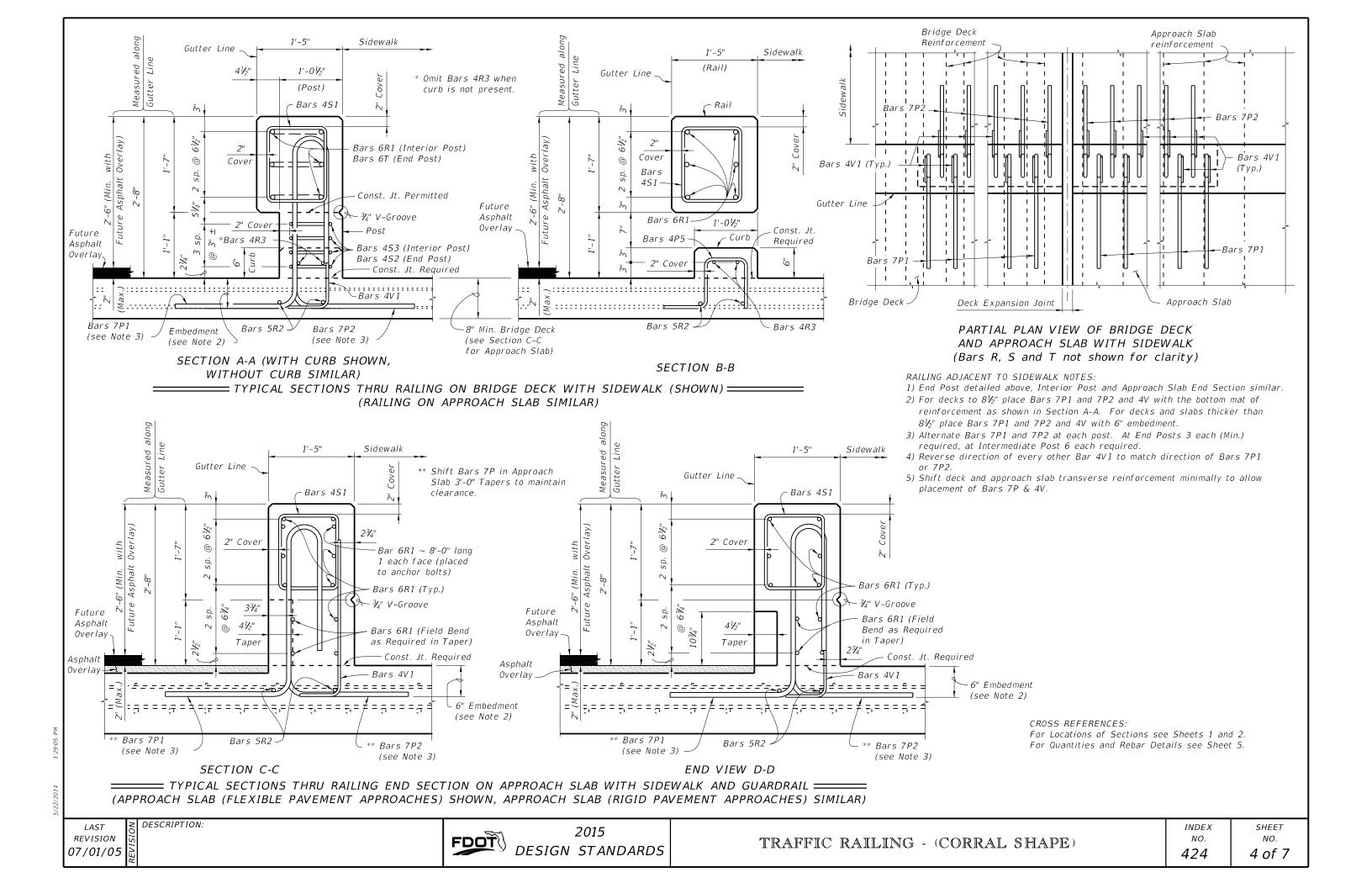
	INDEX	SHEET
AL SHAPE)	NO.	NO.
	423	3 of 3



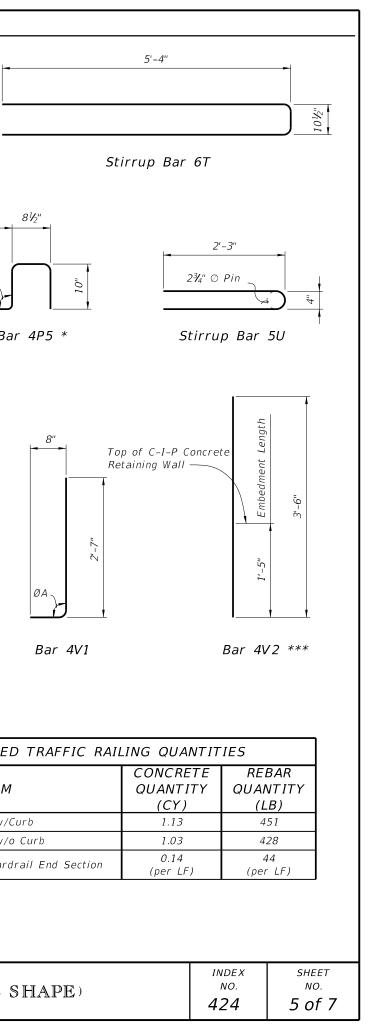


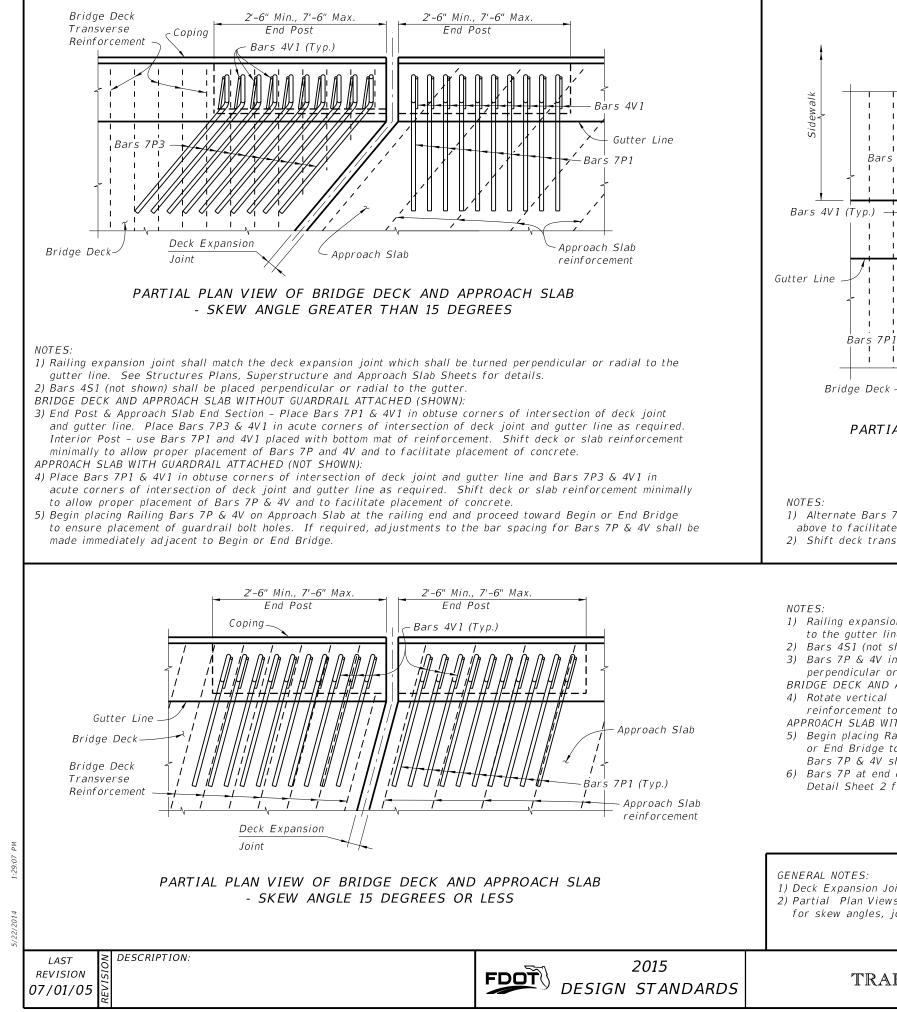


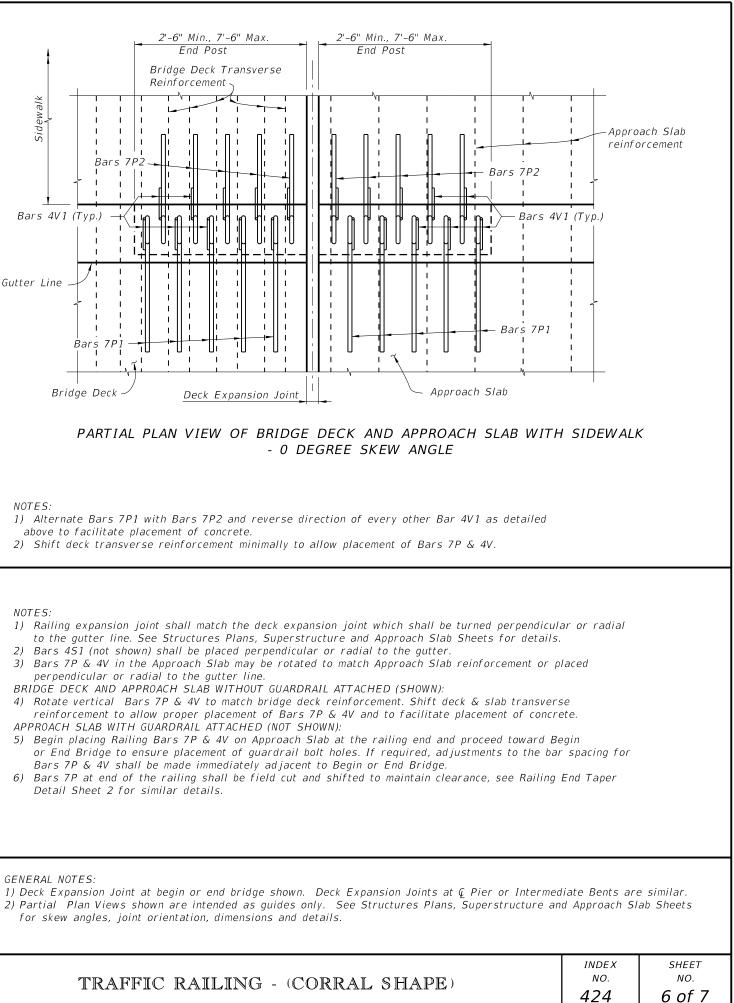
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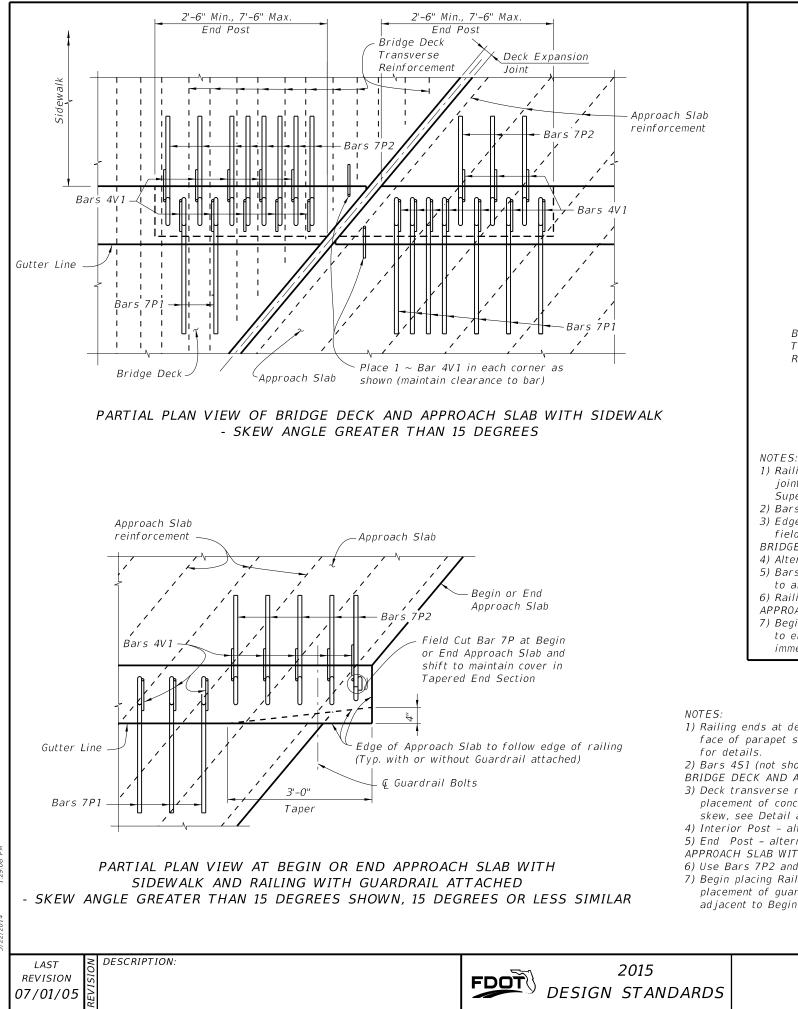


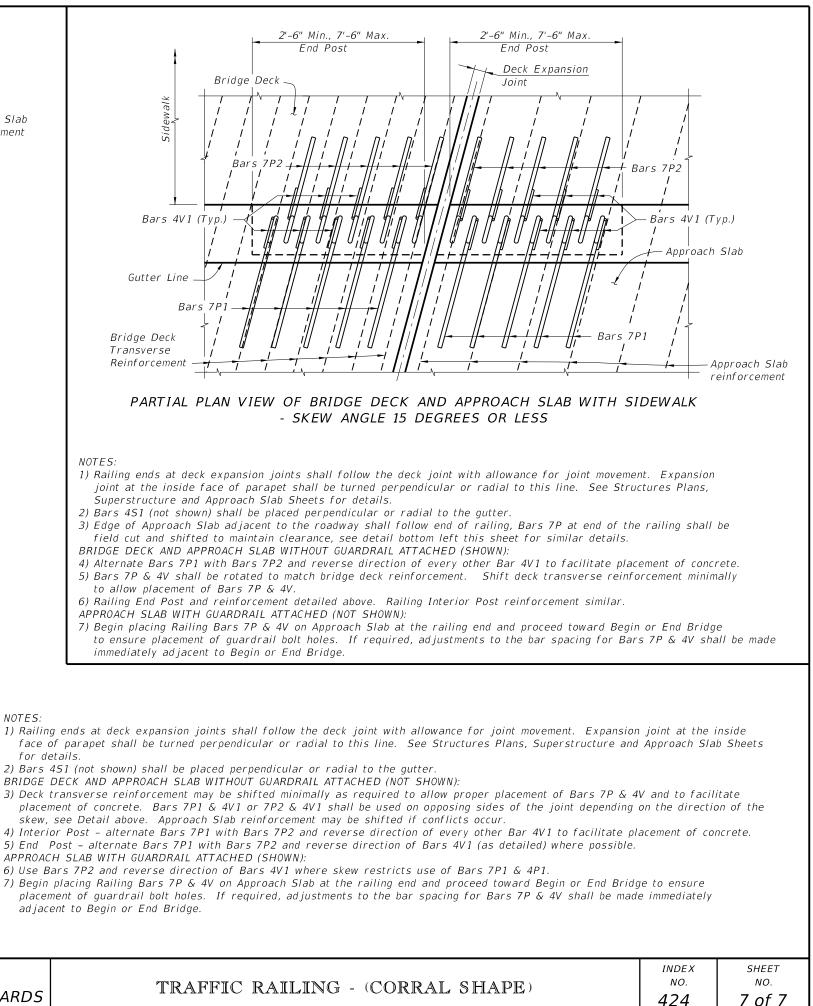
					CONVENTIO	DNAL REINFORG	CING STEEL I	BENDING	DIAGRAMS		
BILL OF	REIN	FORCING	STEEL		1'-0"	8 <sup>1</sup> ⁄₂"	1		4'-8''		
MARK	SIZE	LENGTH	LB/BAR				-				+
P 1	7	7'-4''	15.00							)	
Ρ2	7	7' <i>-3</i> ''	14.82	Bars 6R1, 5R2 & 4R3 *			X. Min.			J	81/2"
Р3	7	7'-2"	14.65	Length As Required	13"		2'-2". Max.				<b>_</b>
*** P4	7	7'-3"	14.82				<u>es ~</u> 7'-2"		Stirrup Ba	ar 453	
* P5	4	2'-11''	1.94				Varies 7'-				
R1	6	As Reqd.	1.5 (LB/LF)	Bars 6R1, 5R2 & 4R3	Stirrup Bar 4S1		¥_				. 8"
R2	5	As Reqd.	1.04 (LB/LF)			Stirrup Bar	452				-
* R3	4	As Reqd.	0.67 (LB/LF)				452		2'-1''		
** <b>5</b> 1	4	5'-0''	3.34						2-1		ØA
** 52	4	Varies 6'-3" Min. 16'-3" Max.	Varies 4.18 Min. 10.86 Max.	2'-1" 7"		2'-0" 7"				, <i>Z'-1'</i>	<u> </u>
** 53	4	11'-3''	7.52								D
Т	6	11'-4''	17.02						<u> </u>		
U	5	4'-8''	4.87		5	-			2'-9"		
V 1	4	3'-2''	2.12	<i>"01</i> -		2'-1'					
*** V2	4	3'-6"	2.34	2'	2'-10"			Parallel to	Joint		
		ng Walls.	sed on C-I-P	Bar 7P1	Ba	ar 7P2	Superstructi	3 7"	∠ Bar 7P3 (Re Dimension		
REINFORC 1. All bar			ending diagrams	s are out to out.	SIDEWALK S	IIGH LOW SIDE SIDE ØA ØA			<u> </u>		
2. The rei	nforceme	ent for the r	ailing on a C-I	-P Concrete Retaining Wall 8" deck with ØA = 90°,	0% to 2%	90° 90°					
where a	applicabl	e. If bottom	n horizontal leg	is of Bars 7P1, 7P3 and 4V1 be substituted for Bars 7P1,	2% to 6%	93° 87°	Top of C-I-P		2 <sup>'</sup> -		
7P3 and	d 4V1 as	shown.		nall have a 2" minimum cover	6% to 10%	96° 84°	Concrete Retaining				
unless	otherwis	e noted.			ØA shall be 90° if Cont.	ractor elects	Wall				ESTIMATE
spliced. Bar 6R	Where	bars are sp ' Min. lap ler	oliced provide a	4R3 may be continuous or 2'-6" Min. lap length for 5R2 and a 1'-3" Min. lap	to place Railing Perpen the Deck.	dicular to			Length 4'-10"		ITEN
5. The sk	ew angle	e for Bars 7		rom joint to joint and side ure Sheets for details.					-6" ent Le	Typical 1	0'-0" Section w,
to side,	See 50	uctures riai	is, superstruct	ure sheets for details.					-'- dme.	Typical 1	0'-0" Section w,
									2'-6" Embedment	Approach	Slab with Guar
								Bar 7P4	***		
LAST REVISION 07/01/05	ISIO	RIPTION:				2015 STANDARDS		TRA	FFIC RA	ILING - (	CORRAL

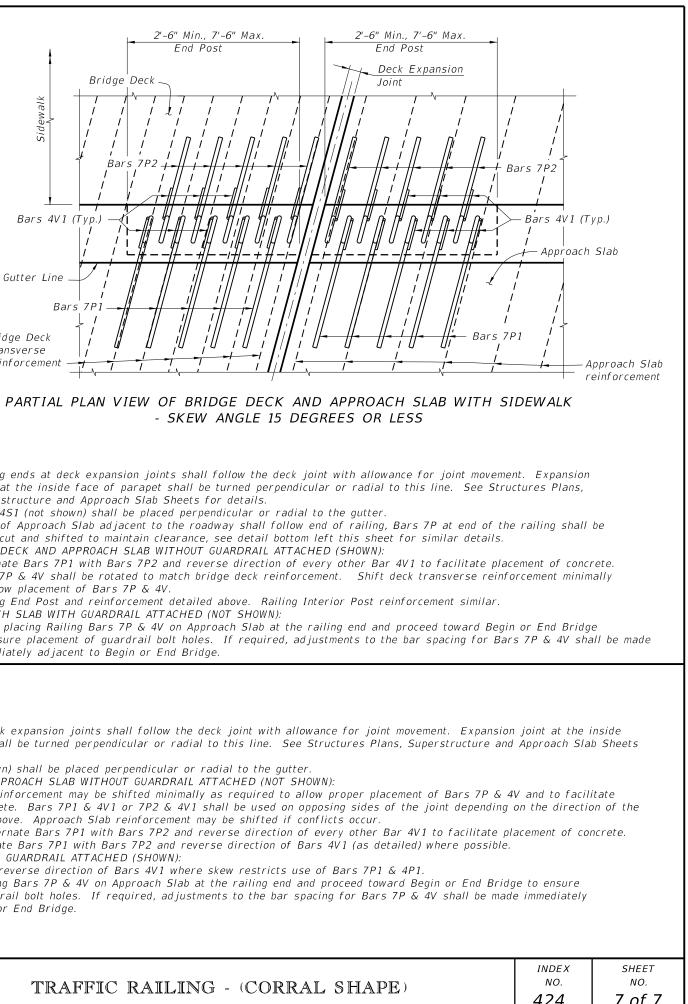








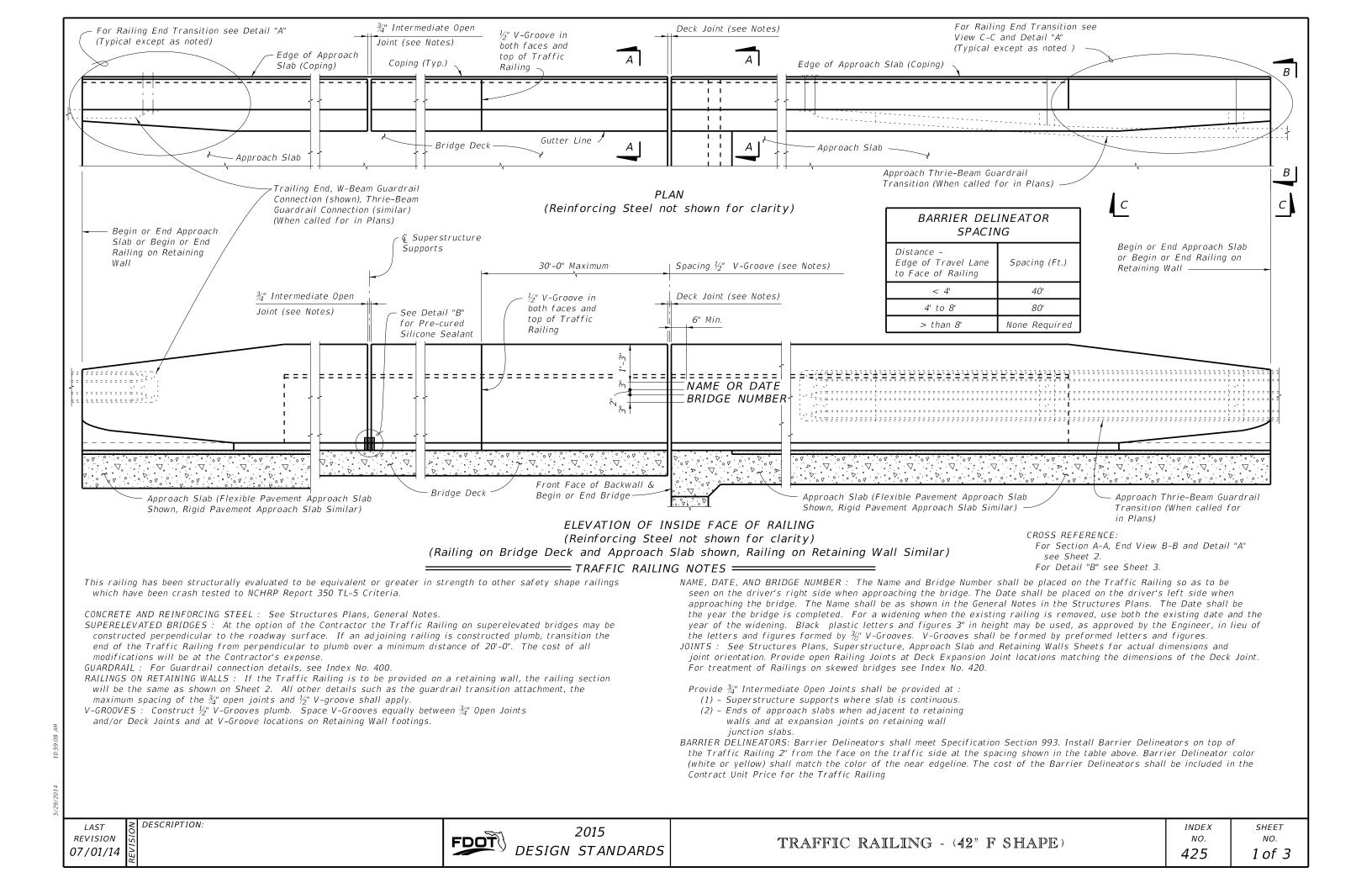


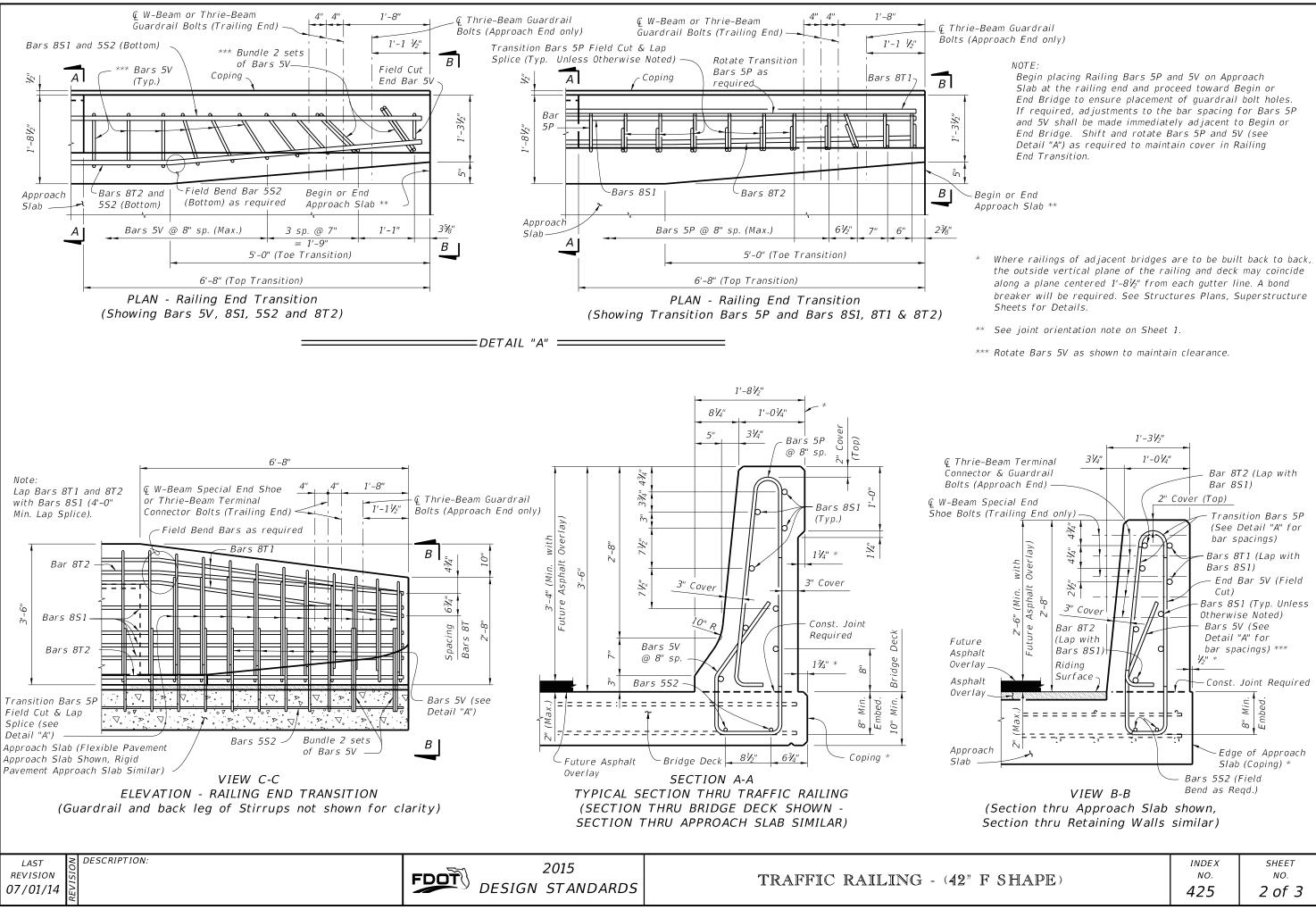


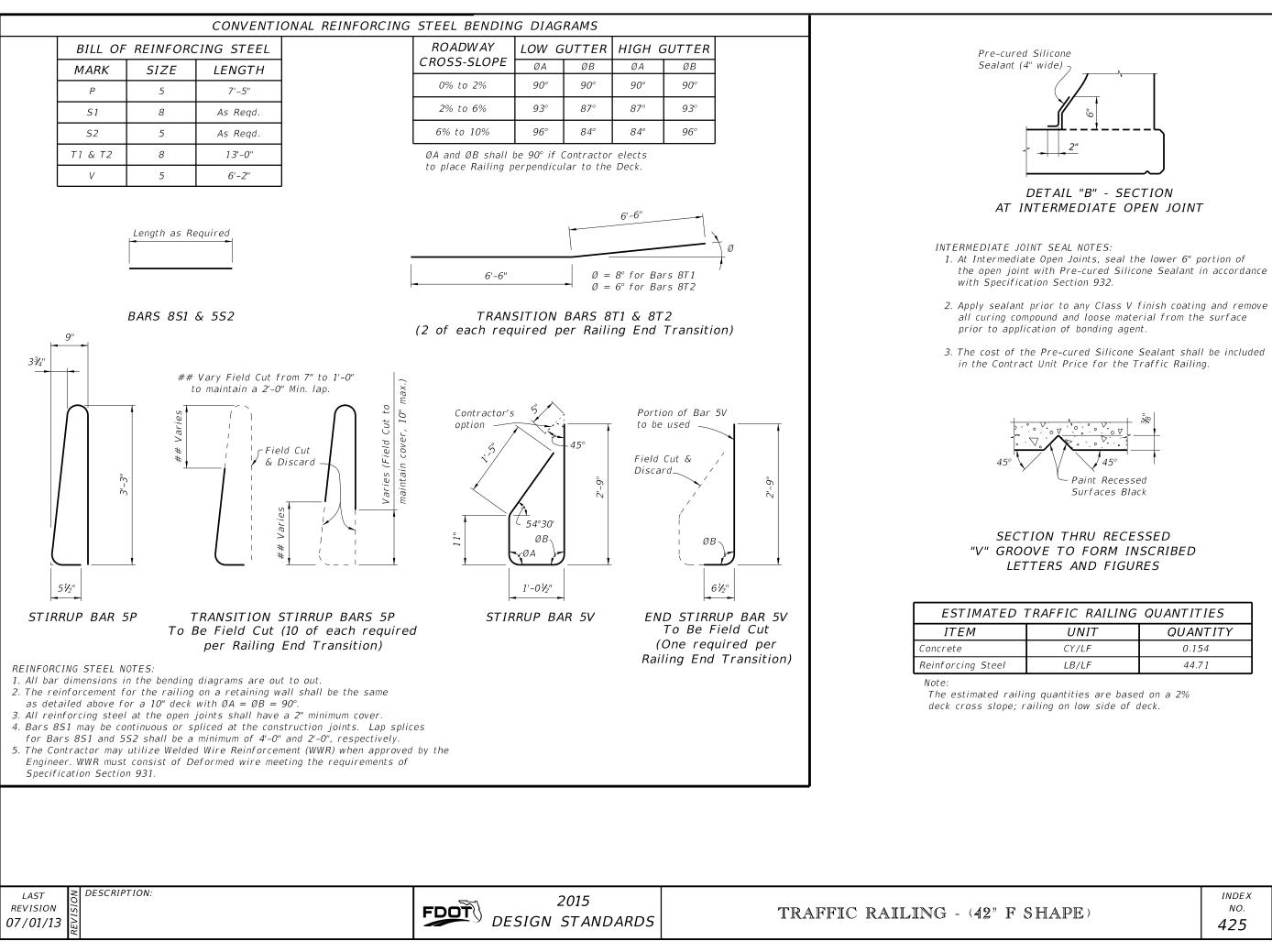
- Superstructure and Approach Slab Sheets for details.

- APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- immediately adjacent to Begin or End Bridge.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (NOT SHOWN):

- APPROACH SLAB WITH GUARDRAIL ATTACHED (SHOWN):
- - adjacent to Begin or End Bridge.







# DETAIL "B" - SECTION

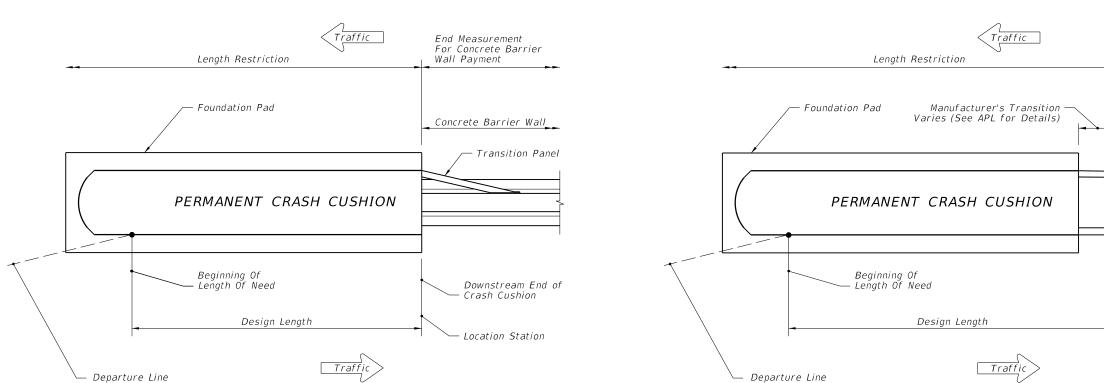
the open joint with Pre-cured Silicone Sealant in accordance

all curing compound and loose material from the surface

3. The cost of the Pre-cured Silicone Sealant shall be included

RAFFIC RAILING	QUANTITIES
UNIT	QUANTITY
CY/LF	0.154
LB/LF	44.71

	INDEX	SHEET
SHAPE)	NO.	NO.
	425	3 of 3



## GENERAL NOTES

- 1. Index 430 is applicable for permanent crash cushion installations that shield the ends of Concrete Barrier Wall or Guardrail, only.
- Design Length is based on a given design speed and the shortest Crash Cushion available on the Approved Products List (APL). When a Length Restriction is not applicable (N/A), then the Contractor 2. has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans. When a Length Restriction is applicable, then the Contractor has the option to select valid Crash Cushions from the APL which have design lengths greater than or equal to the Design Length identified in the plans and that are less than or equal to the Length Restriction identified in the plans.
- 3. For High Speed Facilities with a Design Speed greater than 60 mph, use a TL-3 Crash Cushion.
- Assemble and install Crash Cushions according to the limitations noted on the Approved Products 4. List (APL) webpage, the manufacturer's specifications, and the applicable crash cushion drawings posted on the APL
- When subjected to reverse direction hits, construct Transition Panels from Concrete Barrier Walls 5. to Crash Cushions; for additional details refer to the applicable crash cushion drawings on the APL.
- Galvanize metallic components to meet the requirements for Steel Guardrail, Section 967 of the 6. Standard Specification's for Road and Bridge Construction.
- 7. For Guardrail Applications, construct the Manufacturer's Transition between the Permanent Crash Cushion and the Standard Guardrail Transition; refer to all Standard Guardrail Transition details of this index.
- For additional information on the End Measurement for Guardrail Payment, refer to the Standard 8. Specifications for Road and Bridge Construction, Section 536.
- 9. A yellow Type I Object Marker shall be centered 3' in front of the crash cushion nose. As an option, the contractor may install Retroreflective Sheeting on the nose of the crash cushion. The sheeting to be used must be solid yellow, Type IV or better and must be a product listed on the Department's Approved Products List (APL). The sheeting to be applied to the nose of the crash cushion shall be a minimum of 360 square inches with a minimum height of 15 inches. Mounting hardware, Object Markers or Retroreflective Sheeting shall be in conformance with Section 993 of the Standard Specifications for Road and Bridge Construction.
- 10. The EOR shall provide the station of the Length of Need (LON) location in the plans.

LAST	NC	DESCRIPTION:
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Concrete Barrier Wall Applications

Design Speed

(mph)

35

40

45

50

55

≥ 60

Crash

Test Level

TL-2

TL-3

Design Length

(ft.)

5.75

7.25

7.25

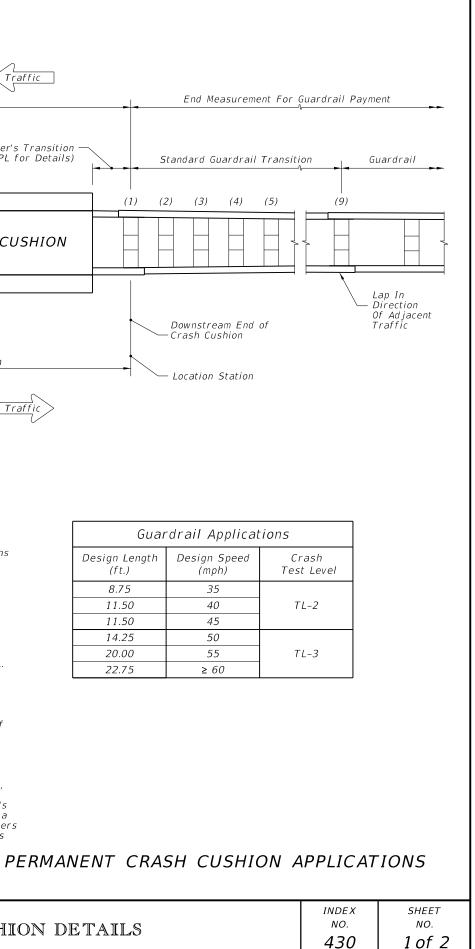
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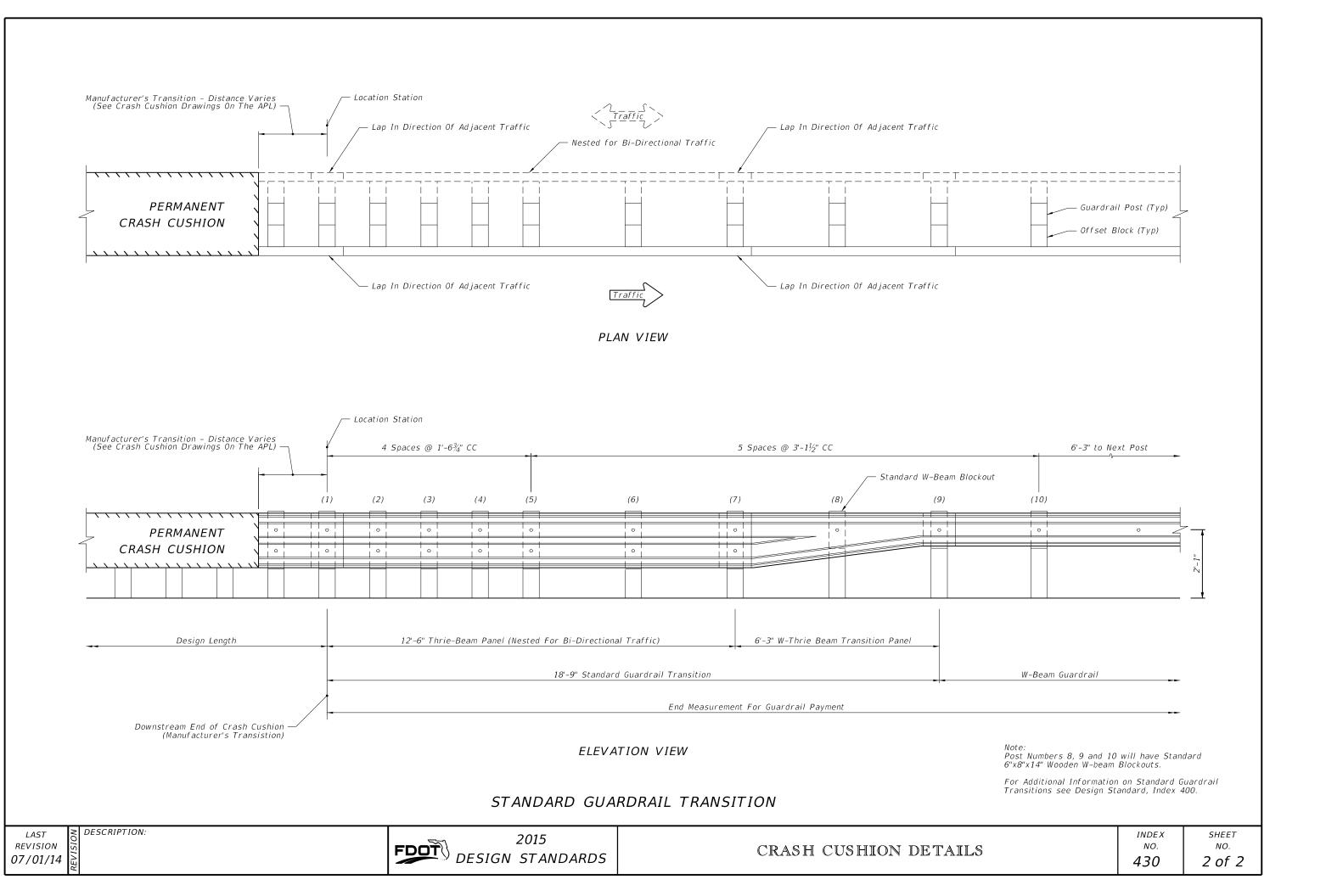
13.25

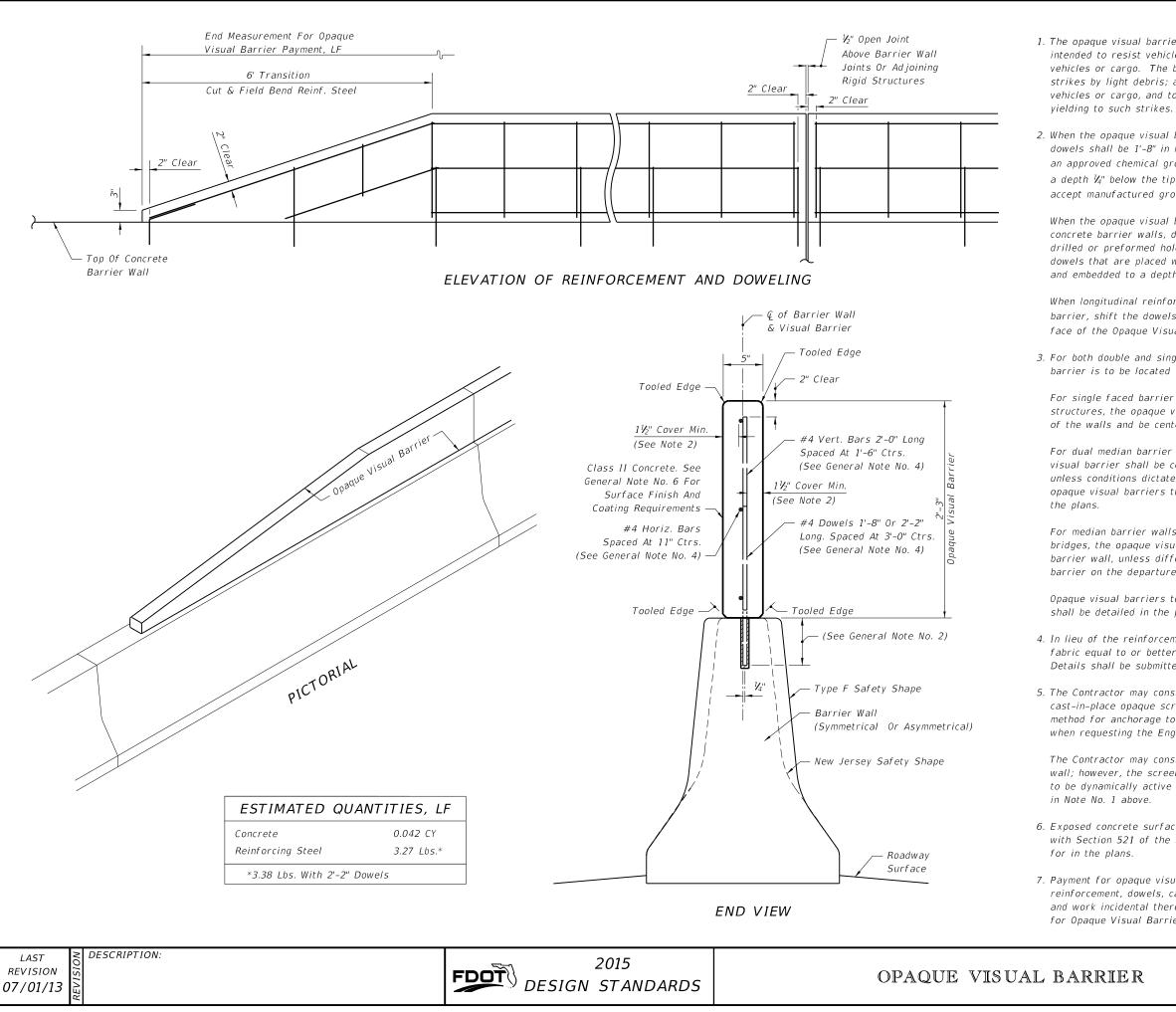
16.00

2015 FOOT DESIGN STANDARDS

CRASH CUSHION DETAILS







# GENERAL NOTES

 The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when yielding to such strikes.

2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved chemical grout. Embedment holes shall be  $\frac{5}{8}$ " diameter, drilled to a depth  $\frac{1}{4}$ " below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the  $1\frac{1}{2}$ " Cover Minimum to the face of the Opaque Visual Barrier.

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

4. In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer. Details shall be submitted with requests for substitution.

5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineer's approval.

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations in Note No. 1 above.

6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called for in the plans.

7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.

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	461	1 of 1

== TRAFFIC RAILING NOTES =====

This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be  $\frac{3}{4}$ " by  $2\frac{1}{2}$ " slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM 449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for  $\frac{7}{6}$ " Ø anchor bolts; 55,000 lbs. for the  $1\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the  $1\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3'' or  $3'-1\frac{1}{2}''$  spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$  slots in three beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in  $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

NEOPRENE PADS: Neoprene pads must be plain pads with a durometer hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required.

ELEVATION MARKERS: Elevation Markers need not be replaced when portions of the existing traffic railing carrying existing elevation markers are removed.

BARRIER DELINEATORS: Barrier Delineators shall conform to Spec. Section 993. Install Barrier Delineators at the top of the guardrail offset blocks at the spacings shown in the table below. Barrier Delineator color (white or yellow) shall conform to the color of the near edgeline.

PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index No. 400 for details.

- BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.
- PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate and Barrier Delineators and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

BARRIER DELII SPACINO	
Distance – Edge of Travel Lane to Face of Railing	
< 4'	
4' to 8'	
> than 8'	/

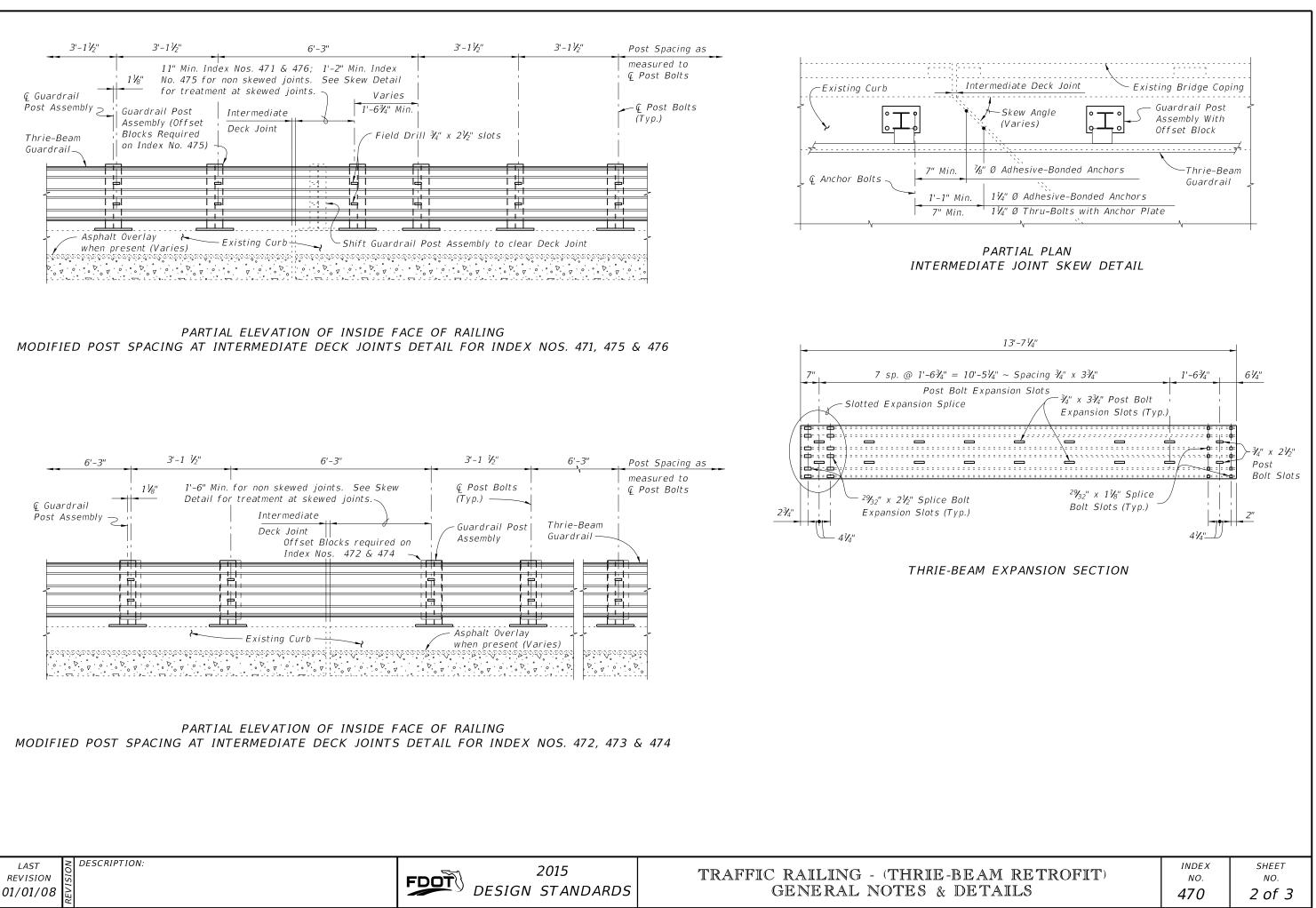
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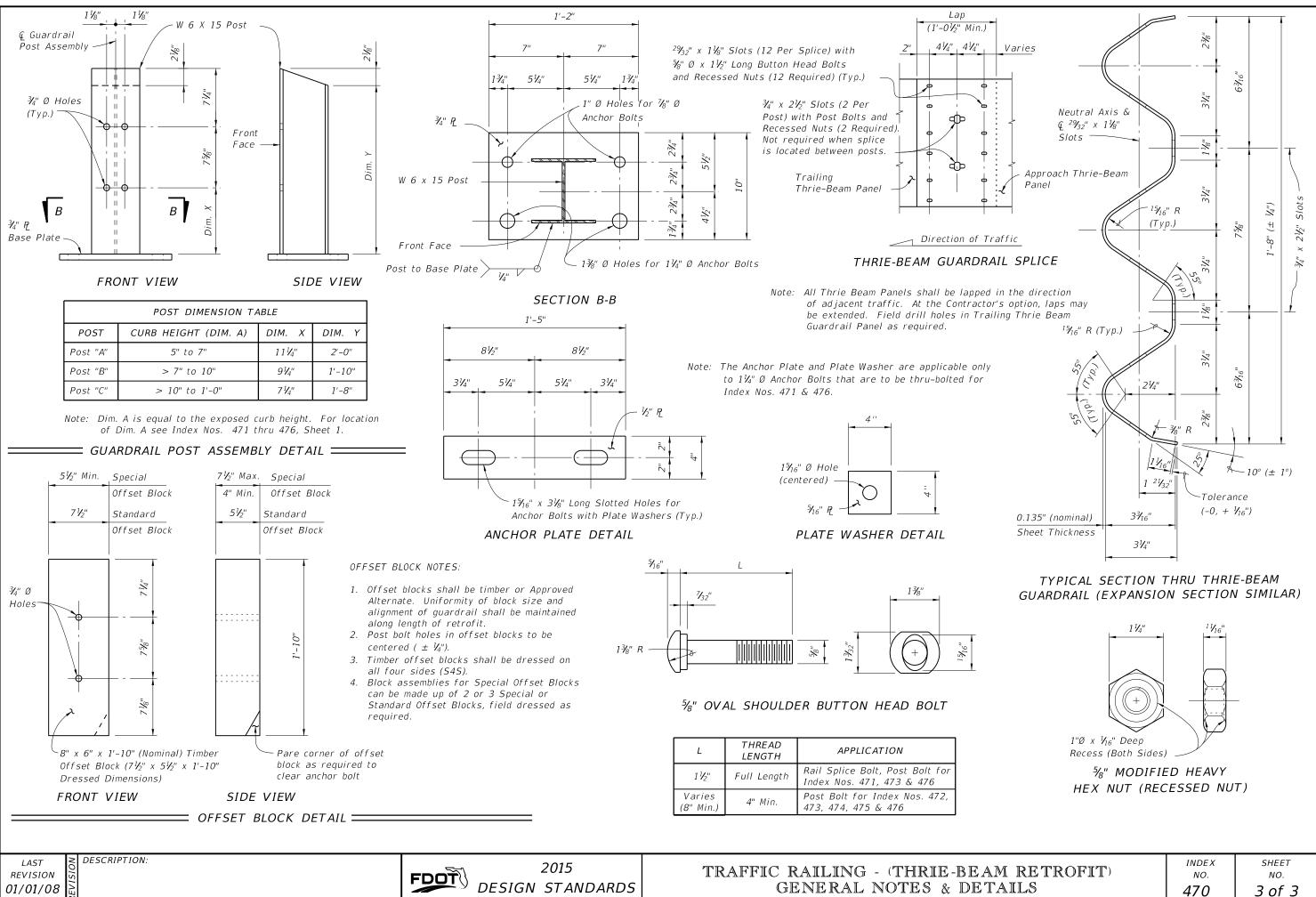
EATOR
Spacing (Ft.)
40'
80'
None Required

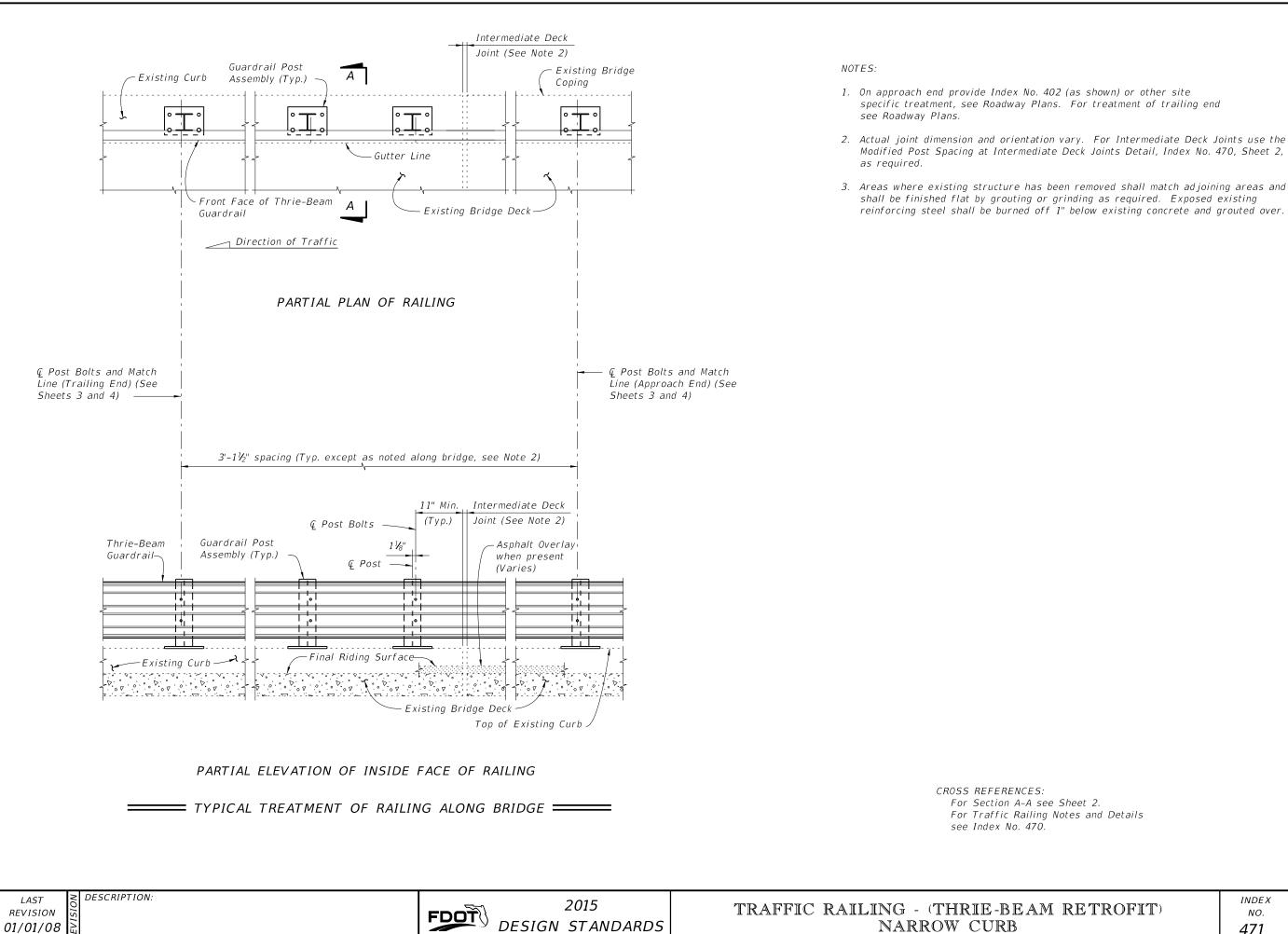
RETROFIT)	INDEX NO.	SHEET NO.
AILS	470	1 of 3



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	DESIGN STANDARD		
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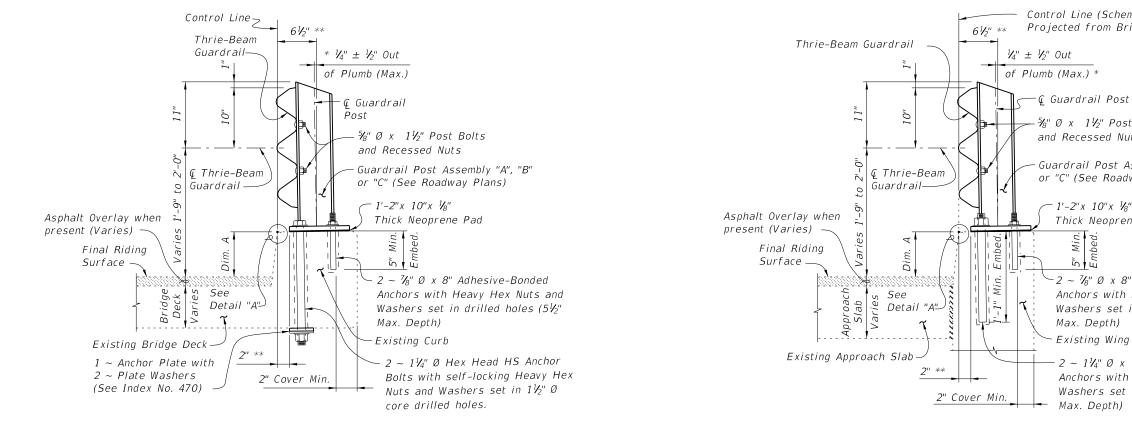


Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2,

shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

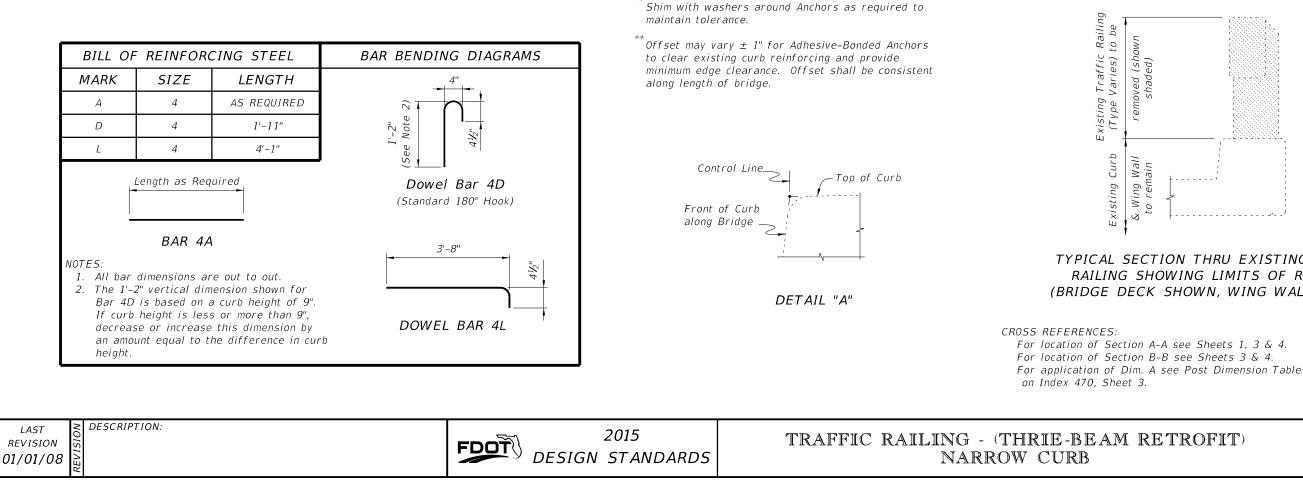
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SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)



Control Line (Scheme 2), Control Line Projected from Bridge (Scheme 3)

 $\frac{5}{8}$ " Ø x 1 $\frac{1}{2}$ " Post Bolts and Recessed Nuts

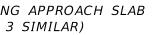
Guardrail Post Assembly "A", "B" or "C" (See Roadway Plans)

 $1'-2'' \times 10'' \times \frac{1}{8}''$ Thick Neoprene Pad

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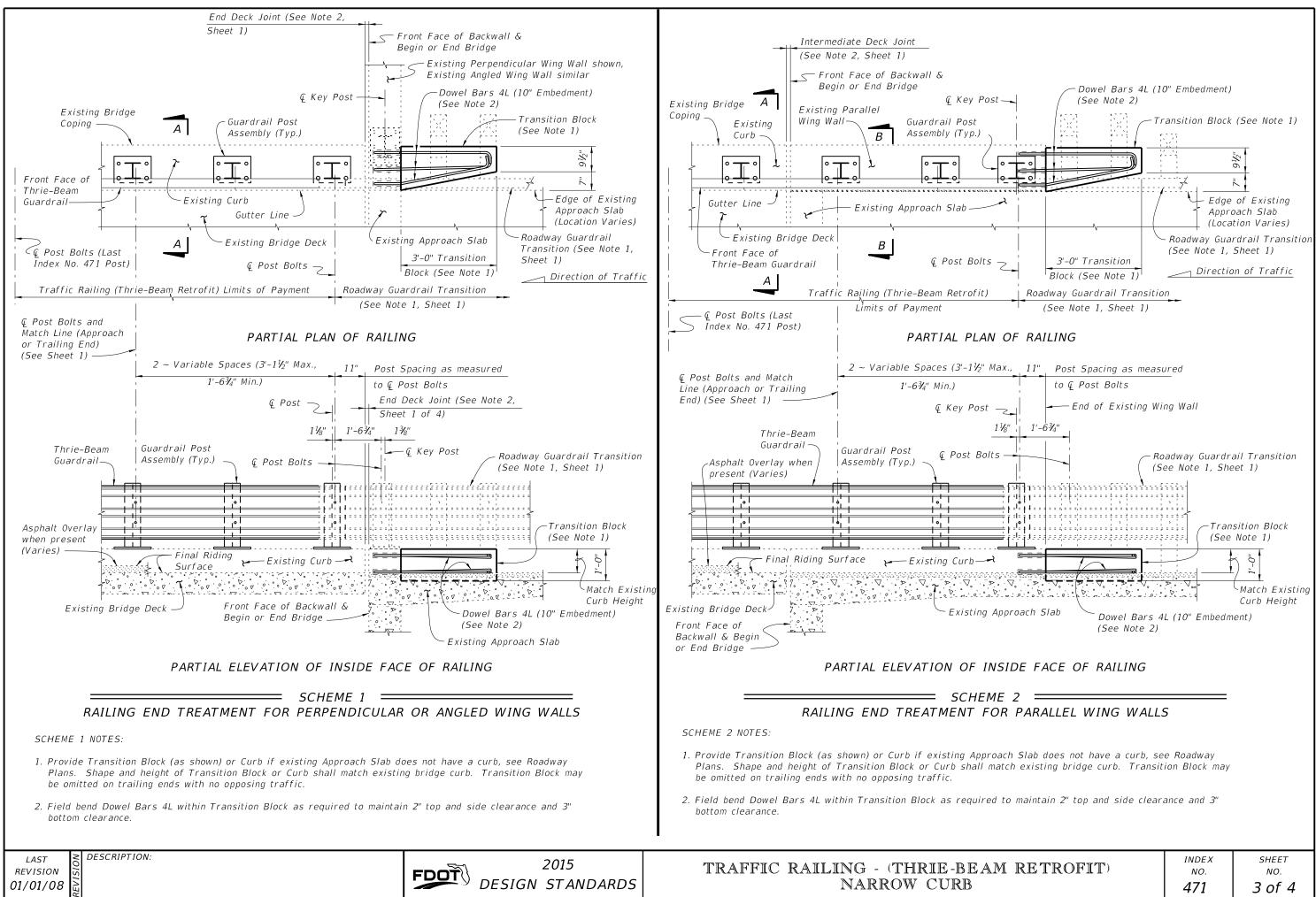
 $-2 \sim \frac{7}{8}$ " Ø x 8" Adhesive-Bonded Anchors with Heavy Hex Nuts and Washers set in drilled holes  $(5\frac{1}{3})$ " Max. Depth) - Existing Wing Wall

 $2 \sim 1 \frac{1}{4}$ " Ø x 1'-4" Adhesive-Bonded Anchors with Heavy Hex Nuts and Washers set in drilled holes  $(1'-1\frac{1}{2}'')$ Max. Depth)

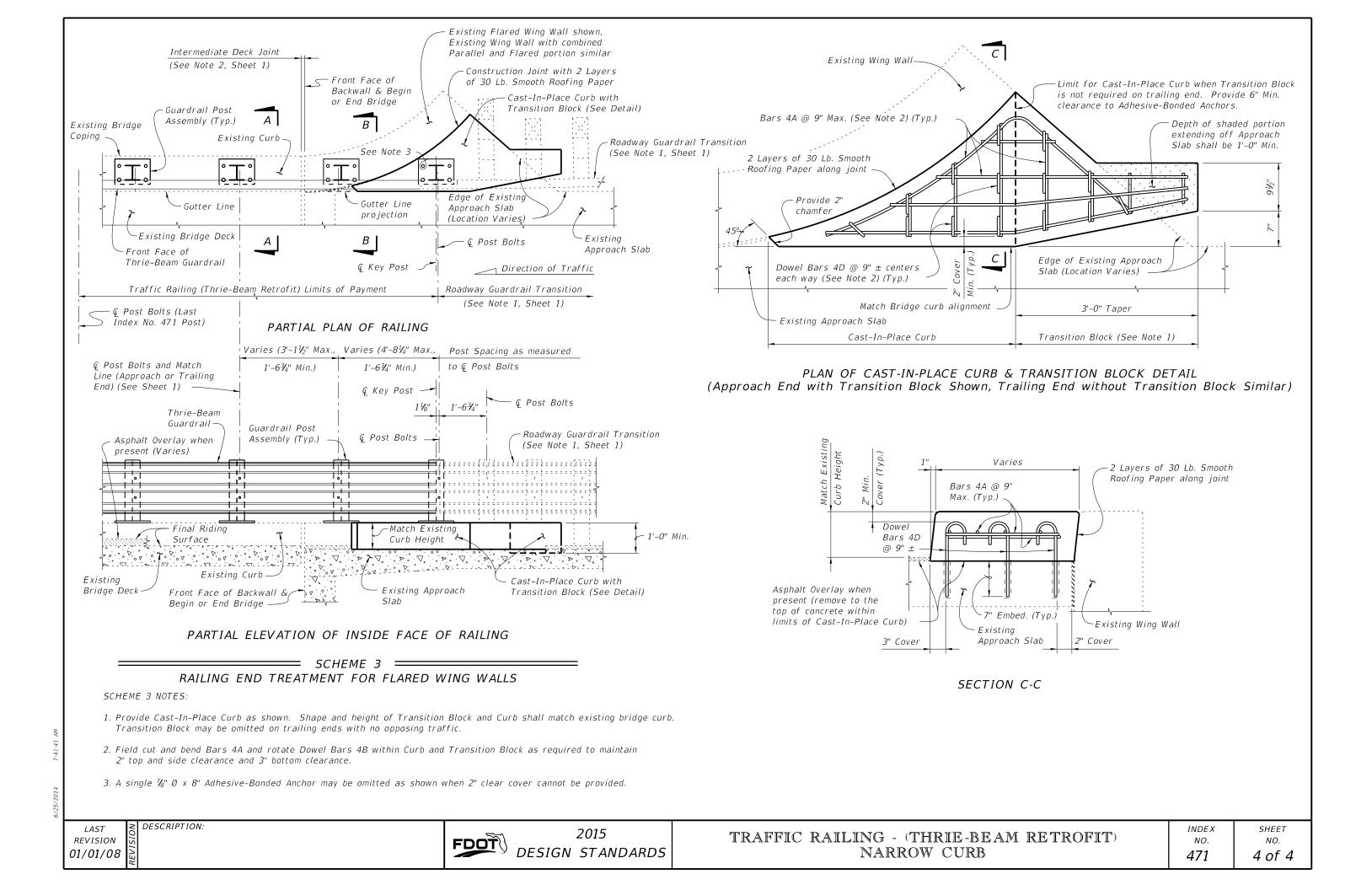


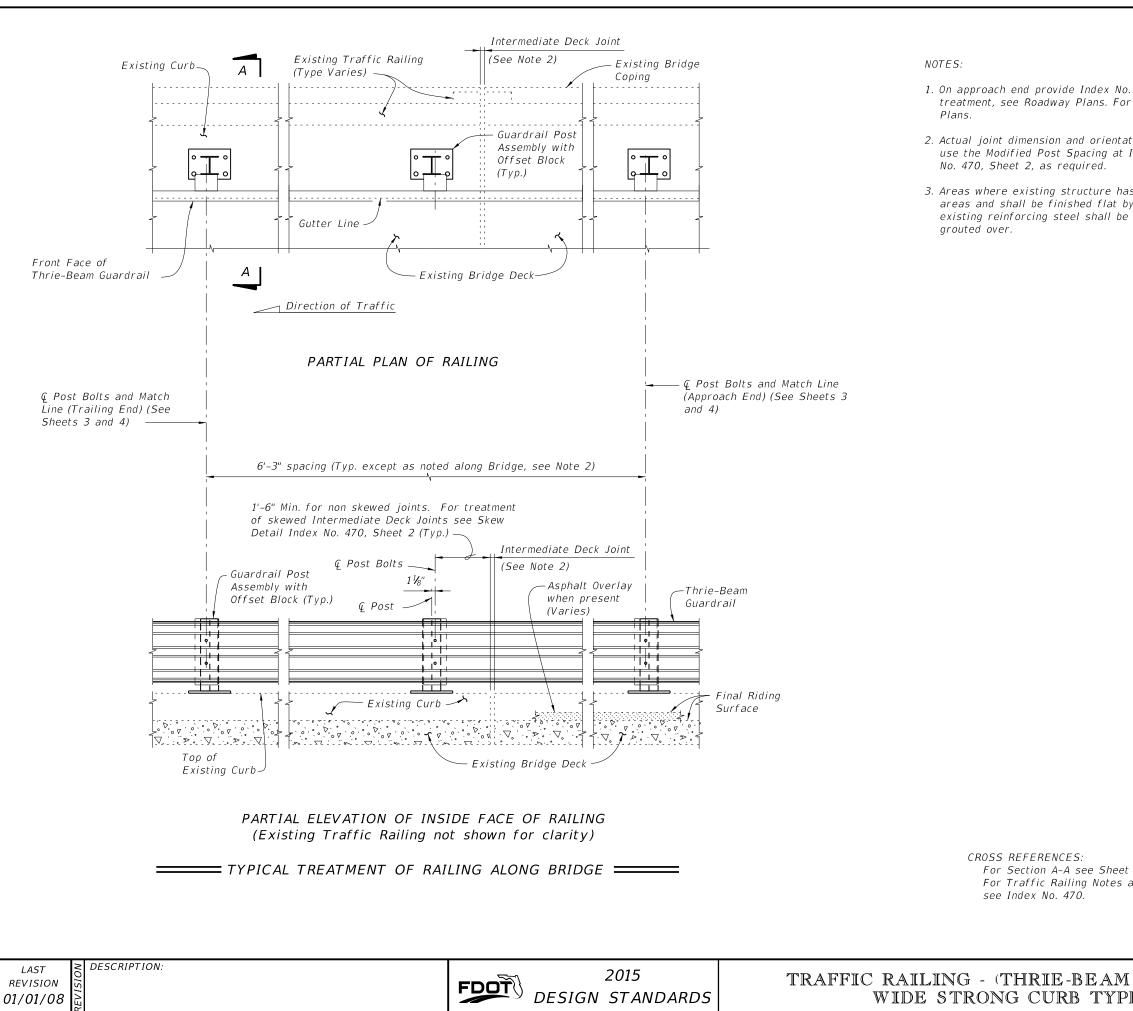
TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

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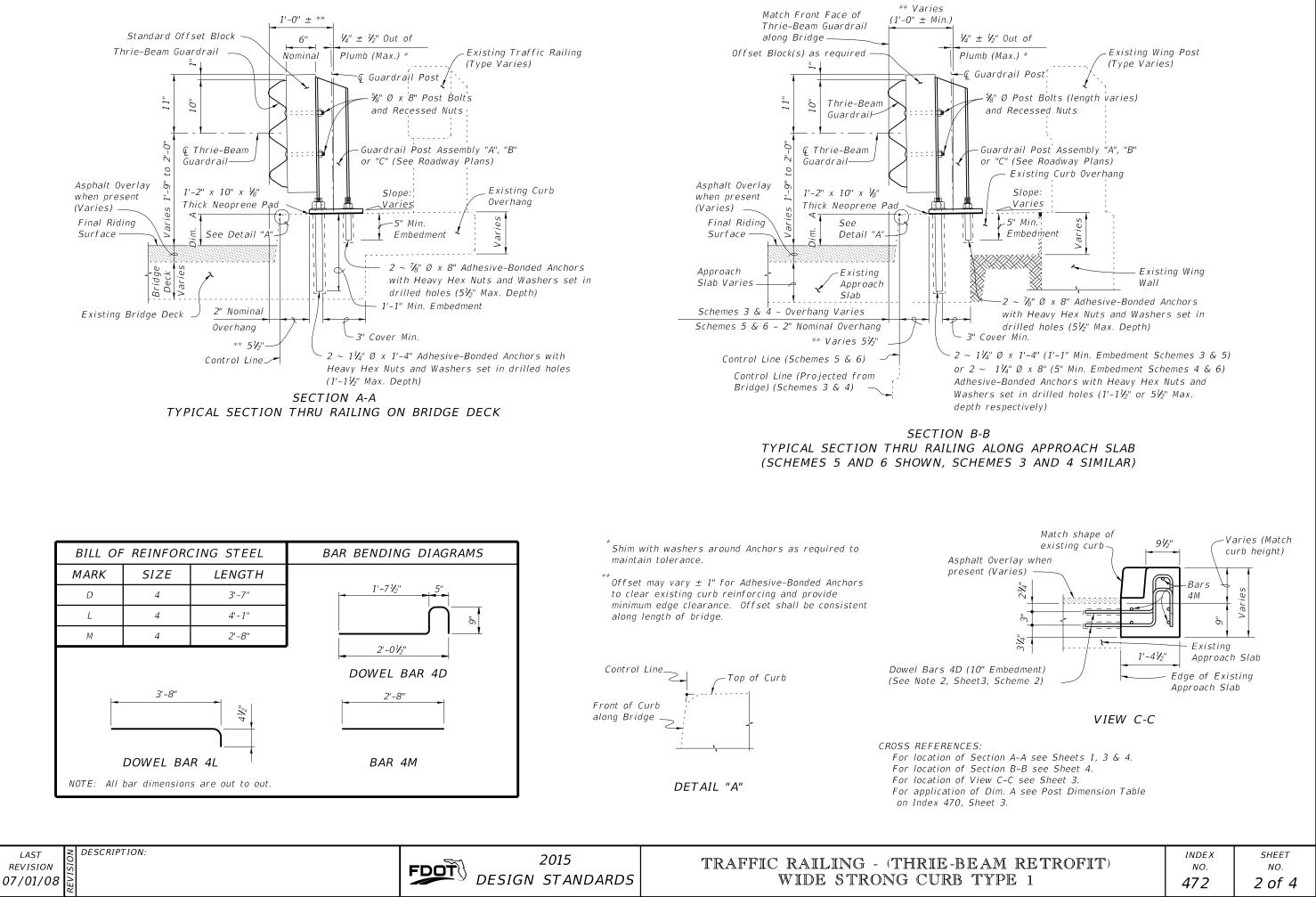
1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway

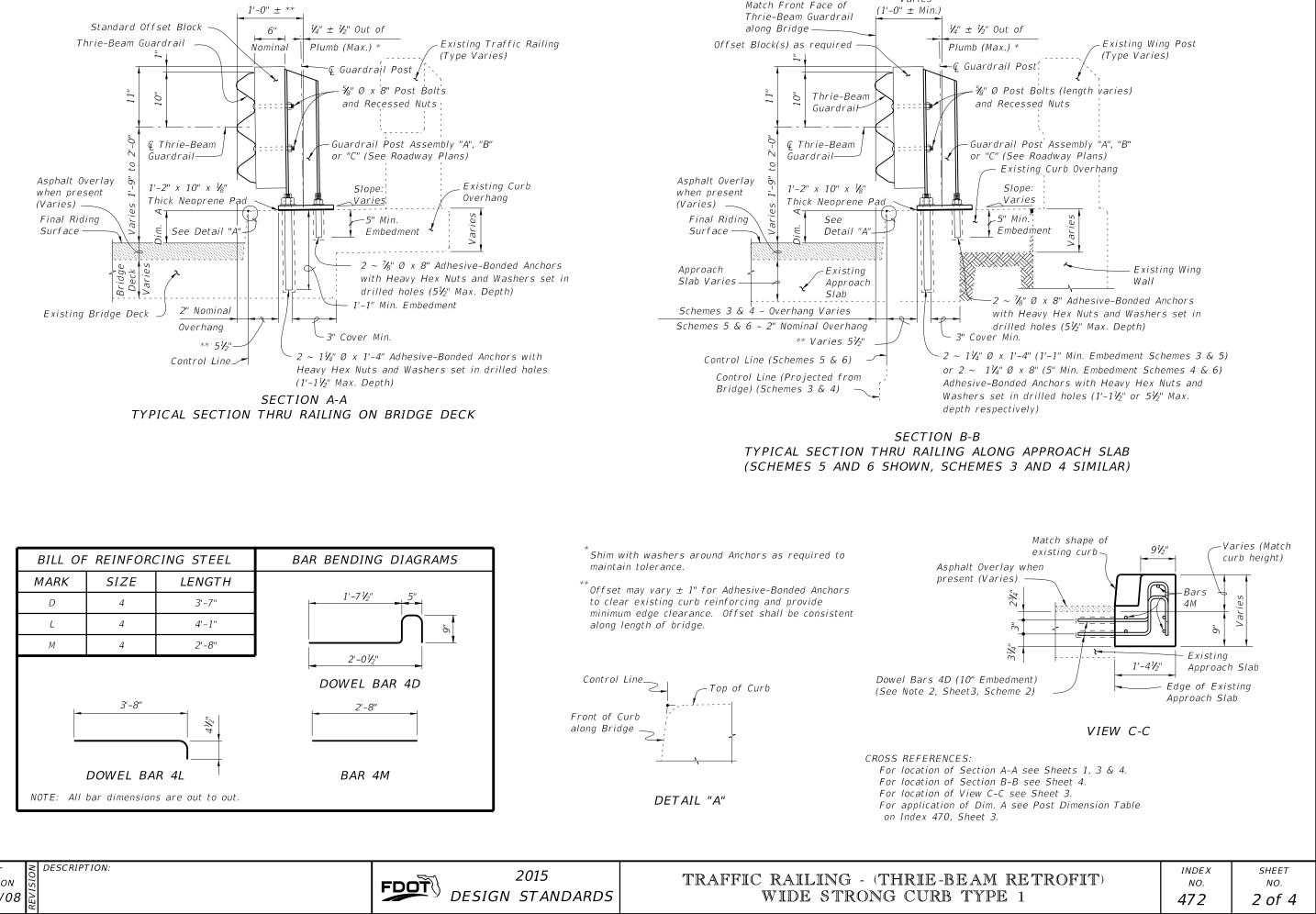
2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index

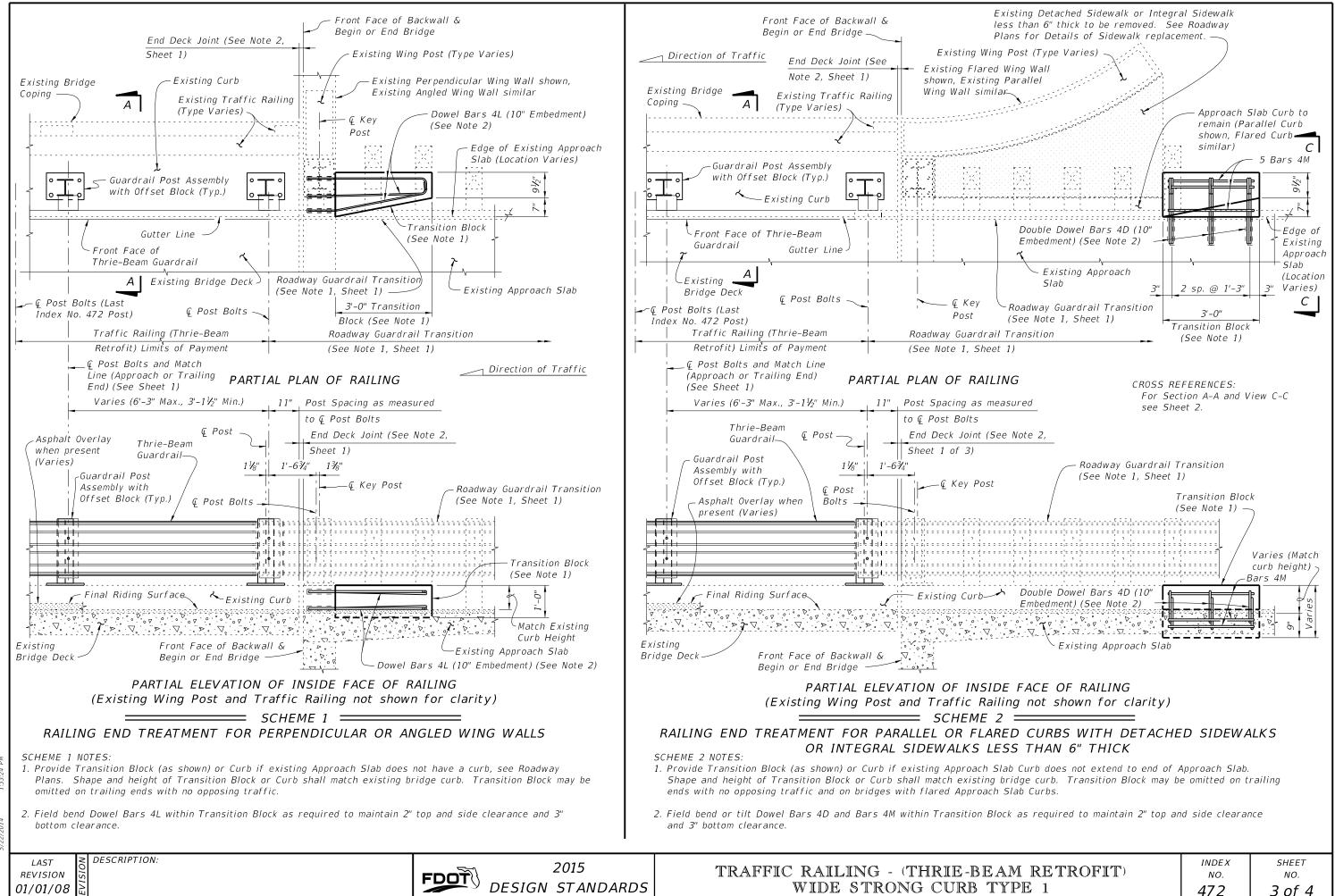
3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and

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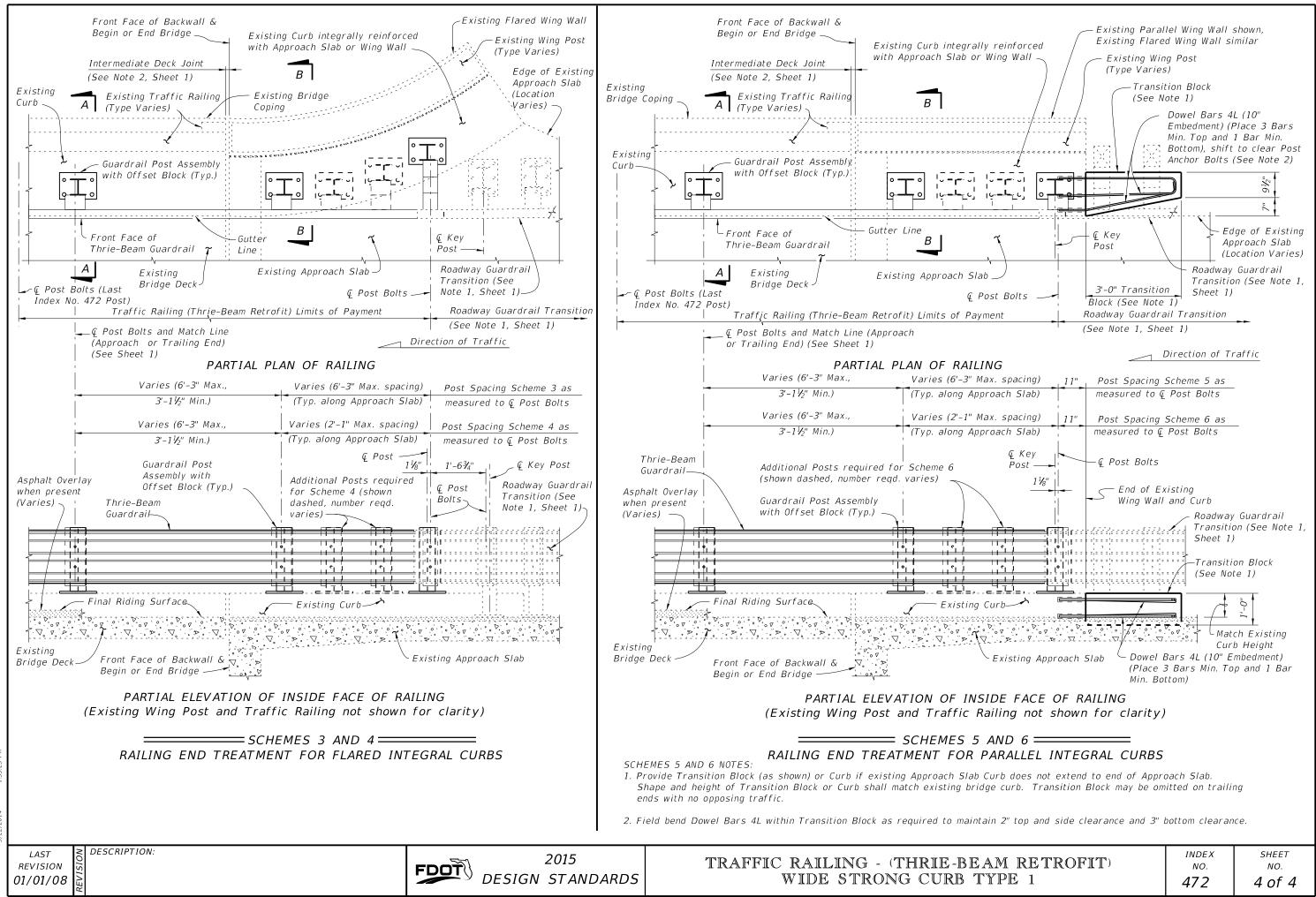
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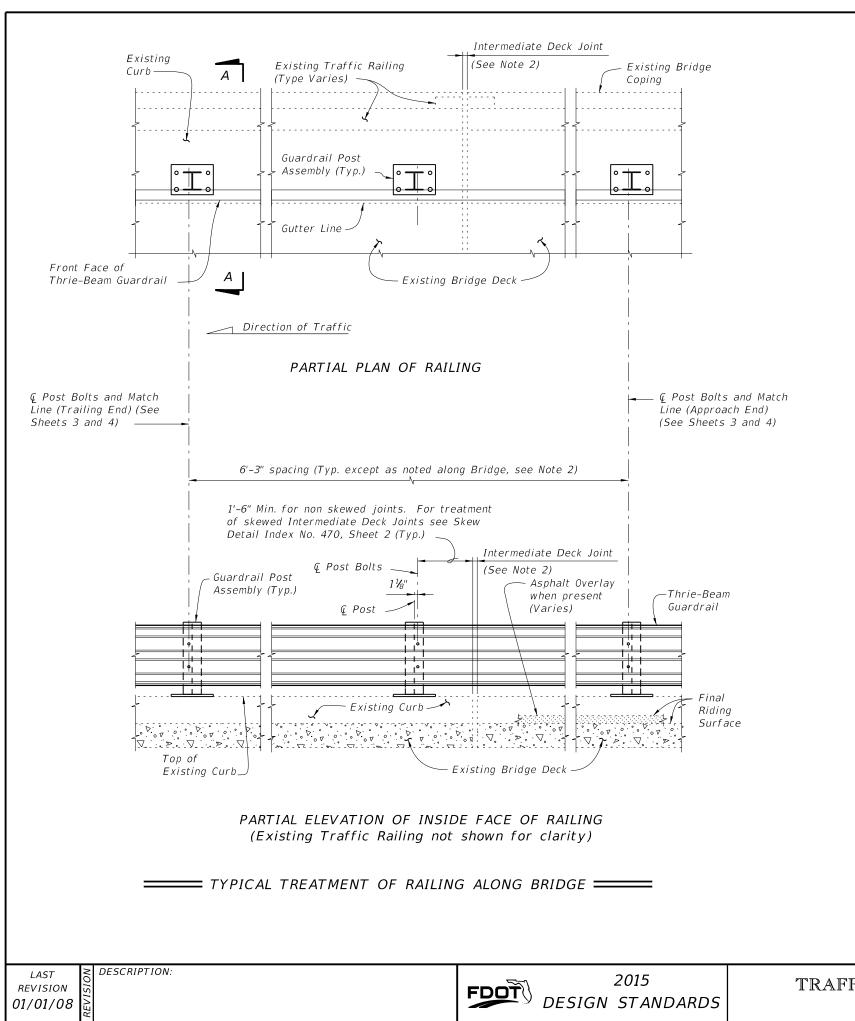




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## NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be be burned off 1" below existing concrete and grouted over.

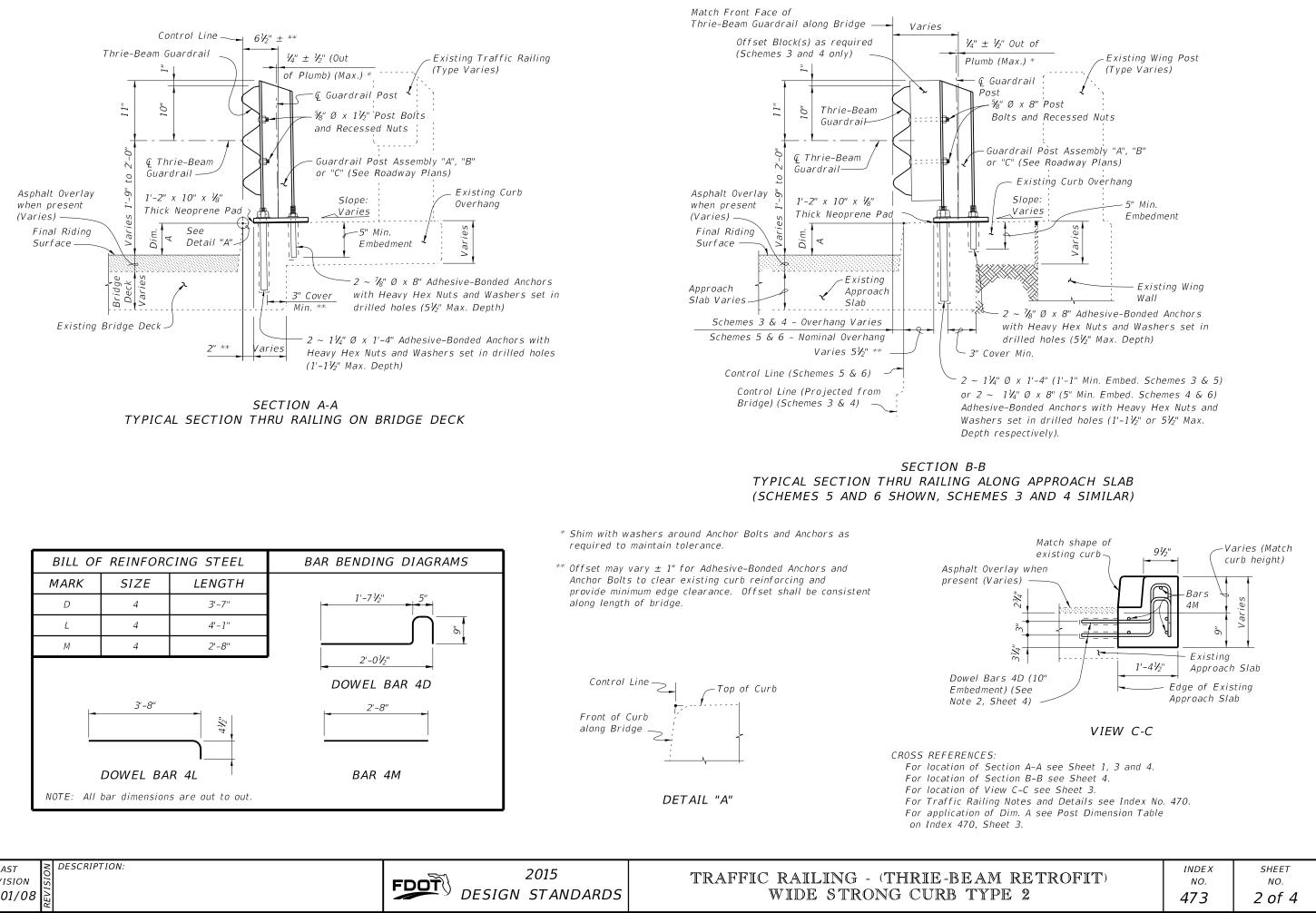
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

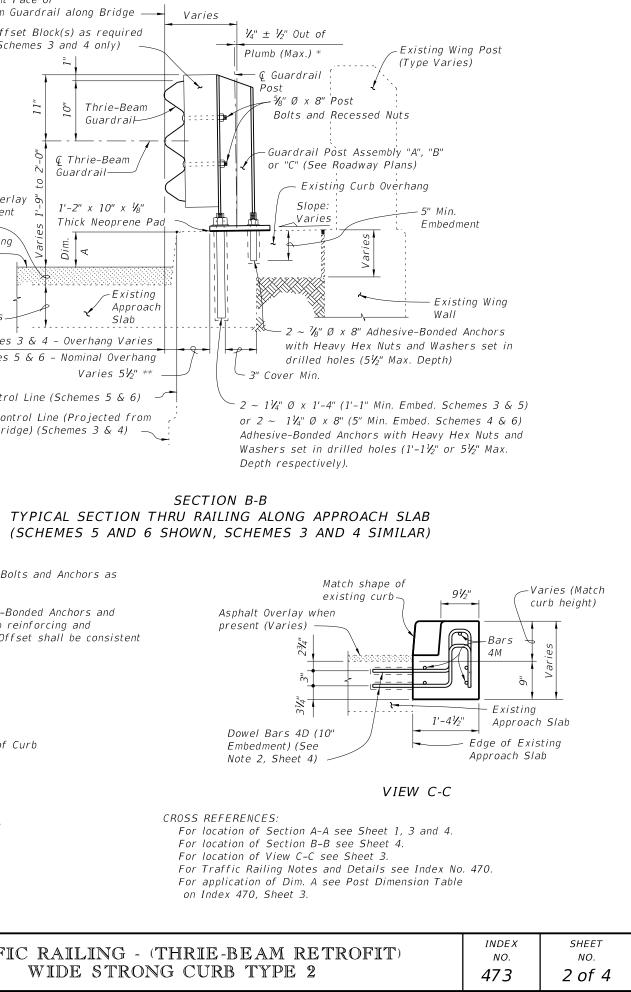
TRAFFIC RAILING - (THRIE-BEAN WIDE STRONG CURB TY

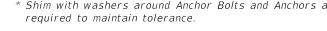
2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified

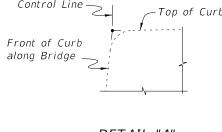
finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall

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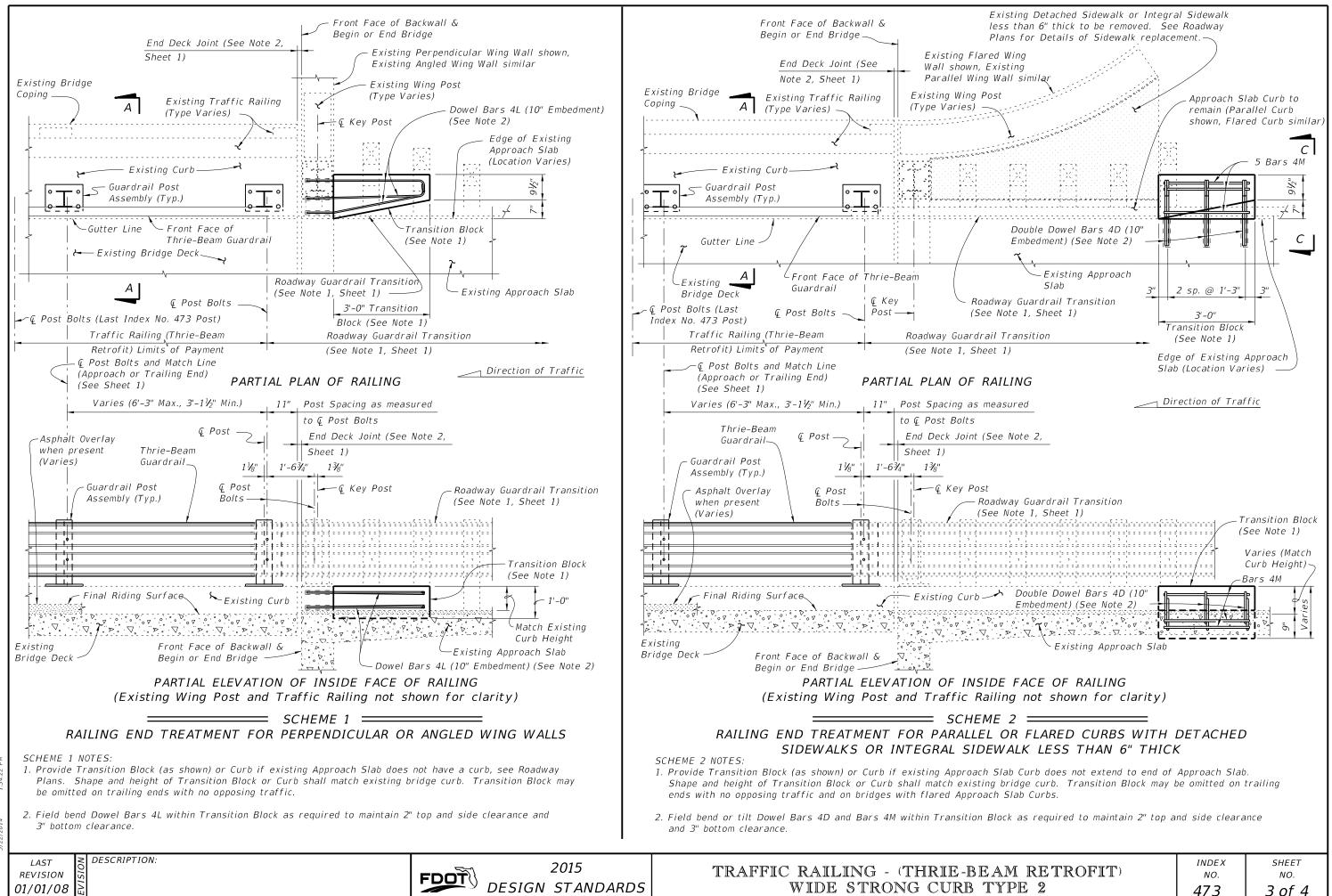




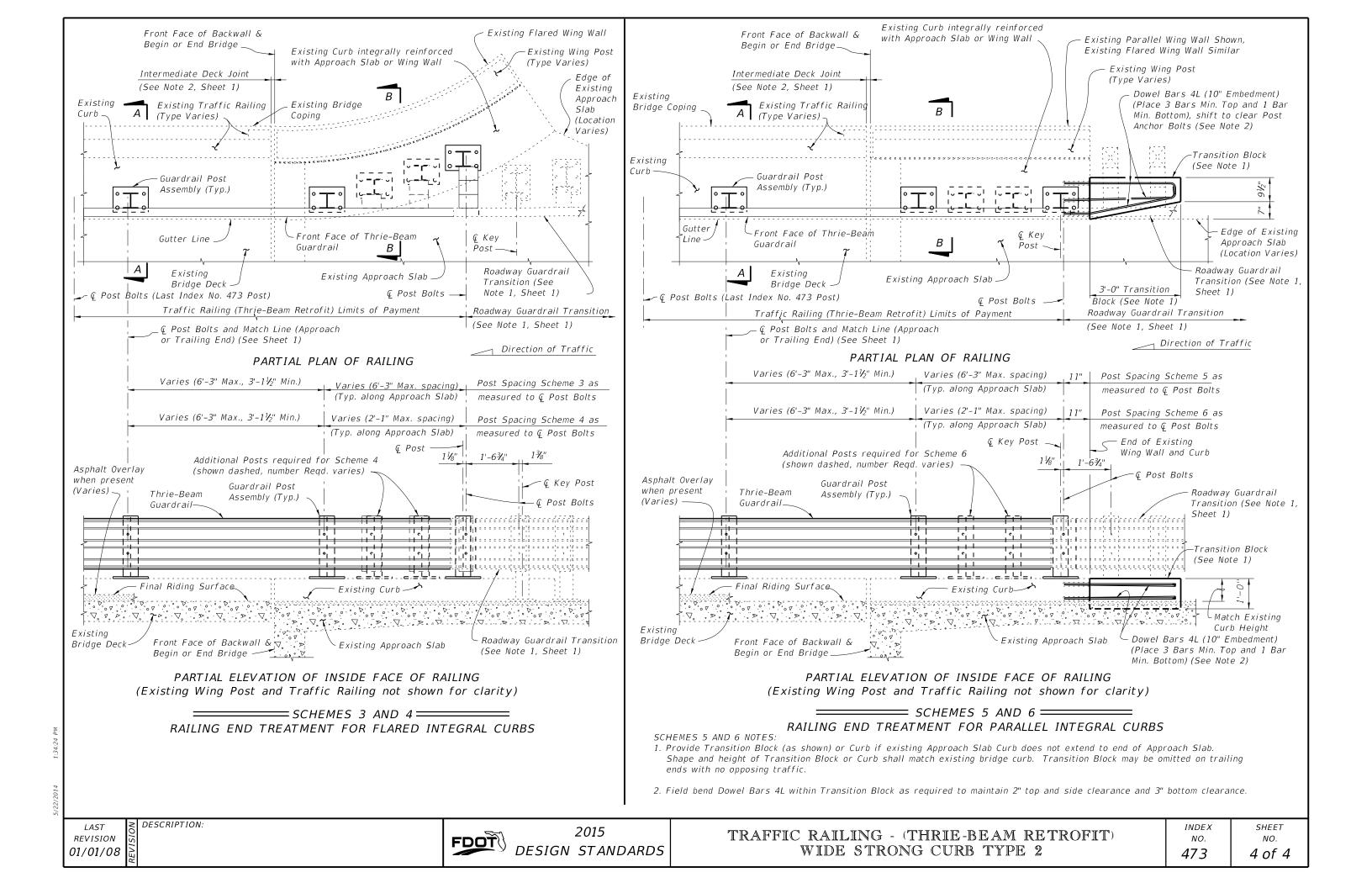


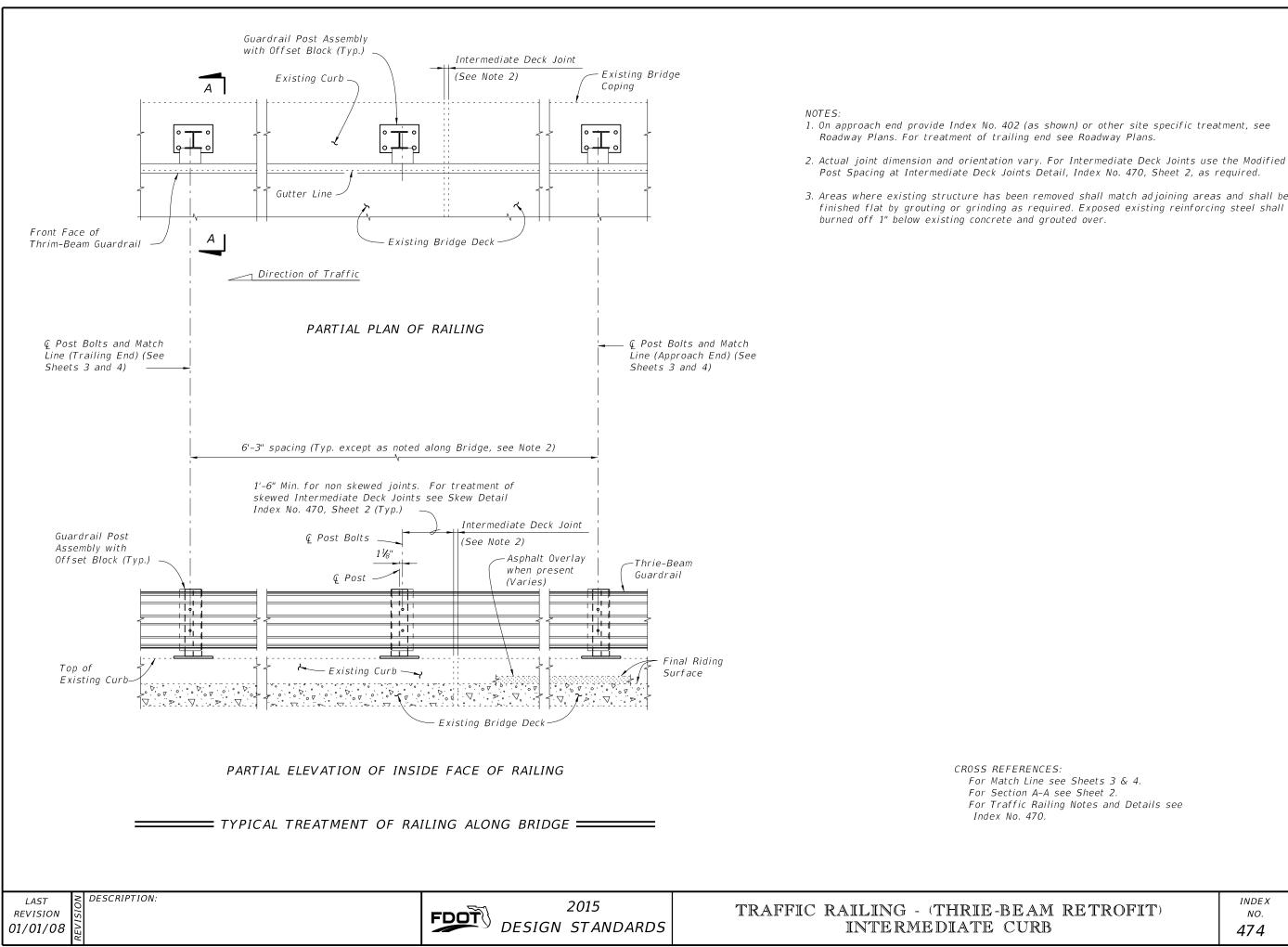


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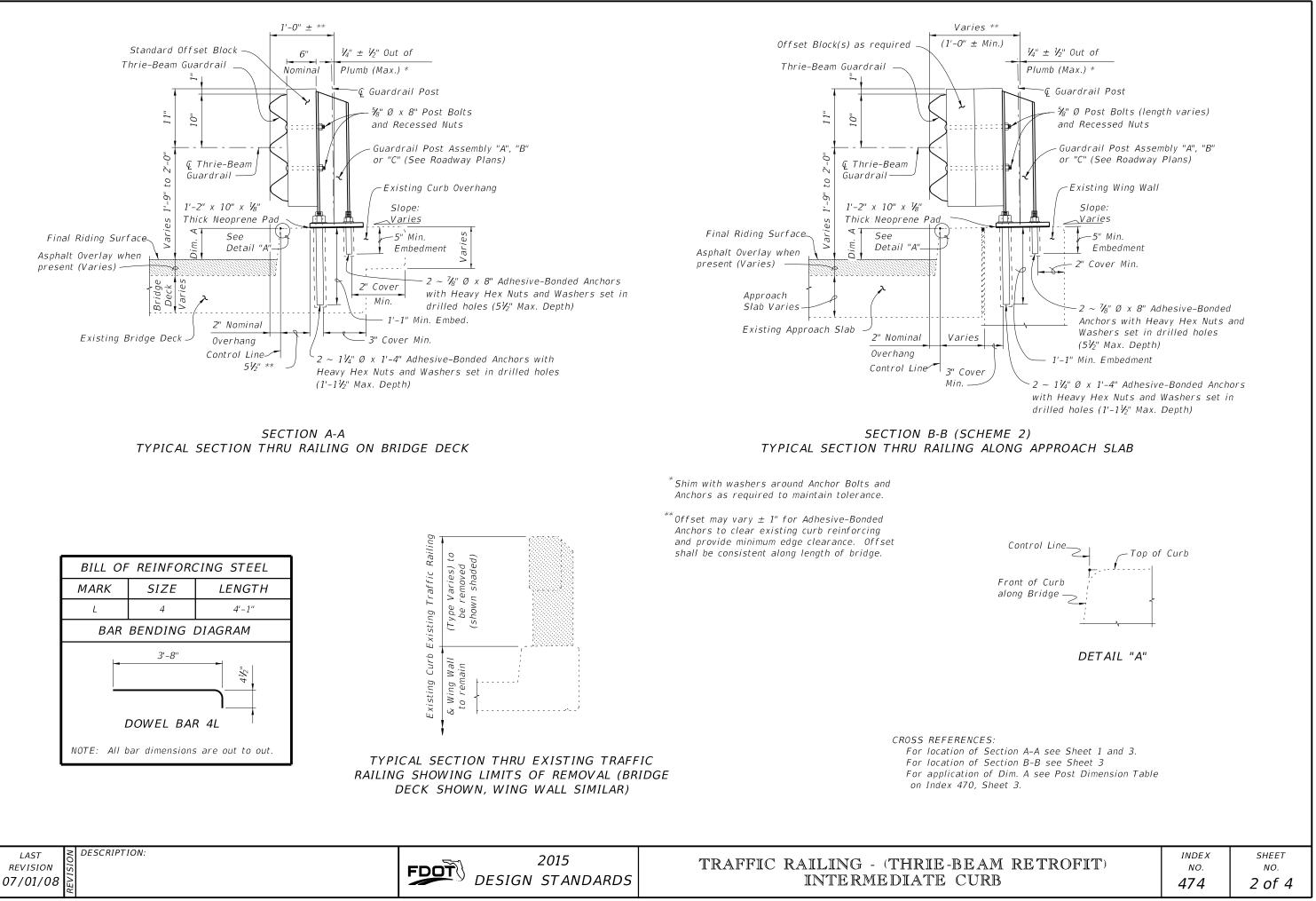


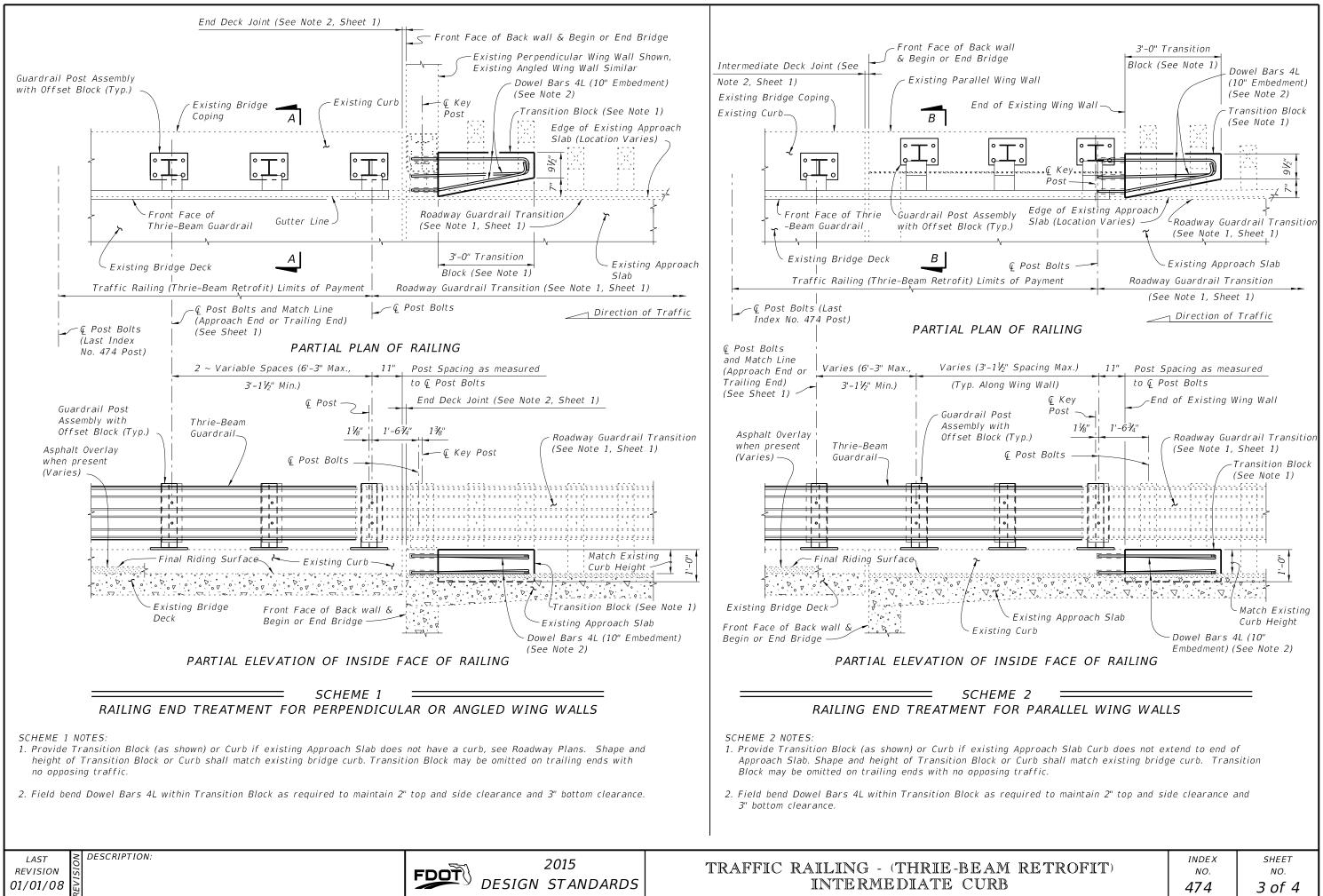


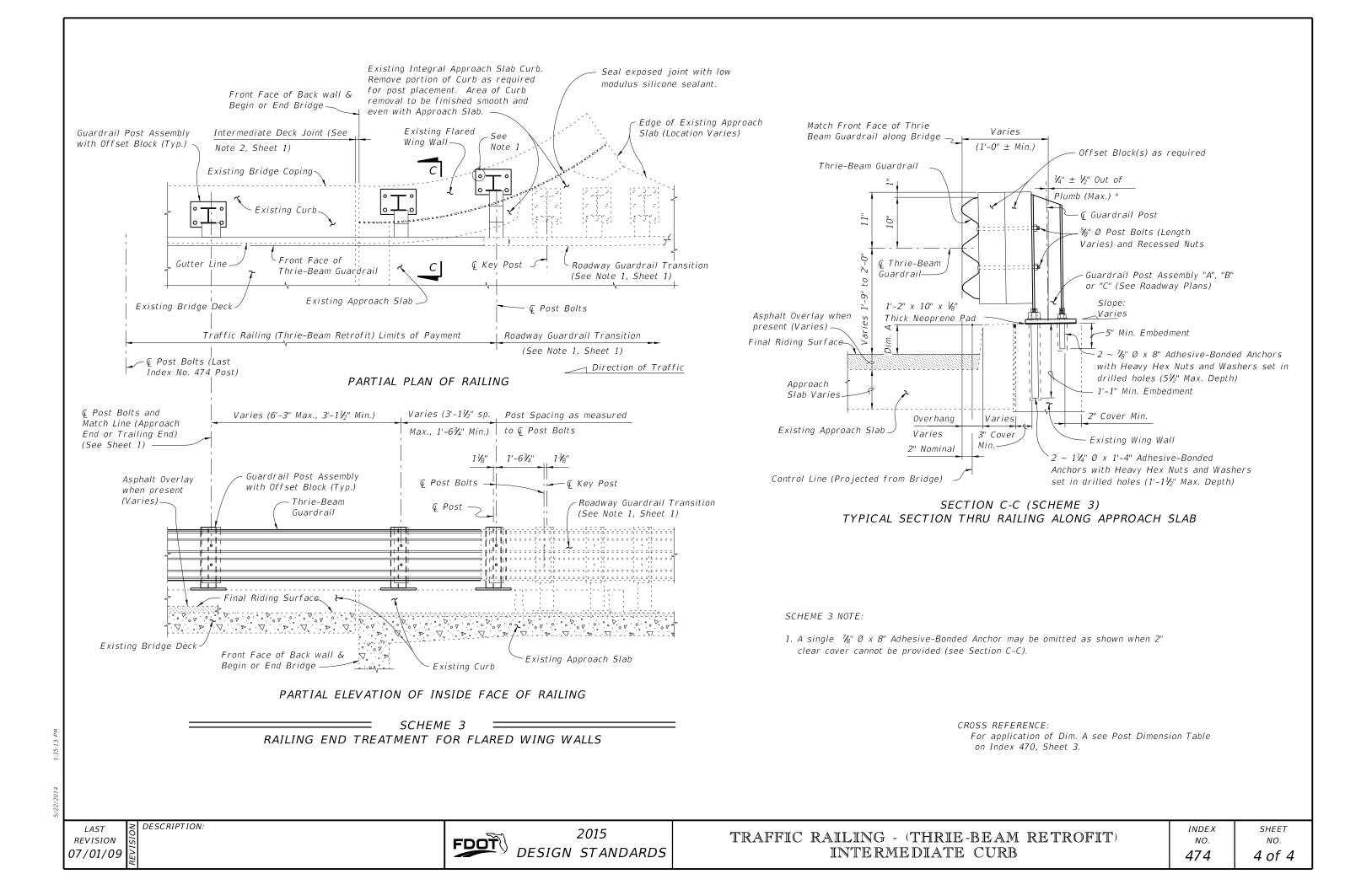
3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be

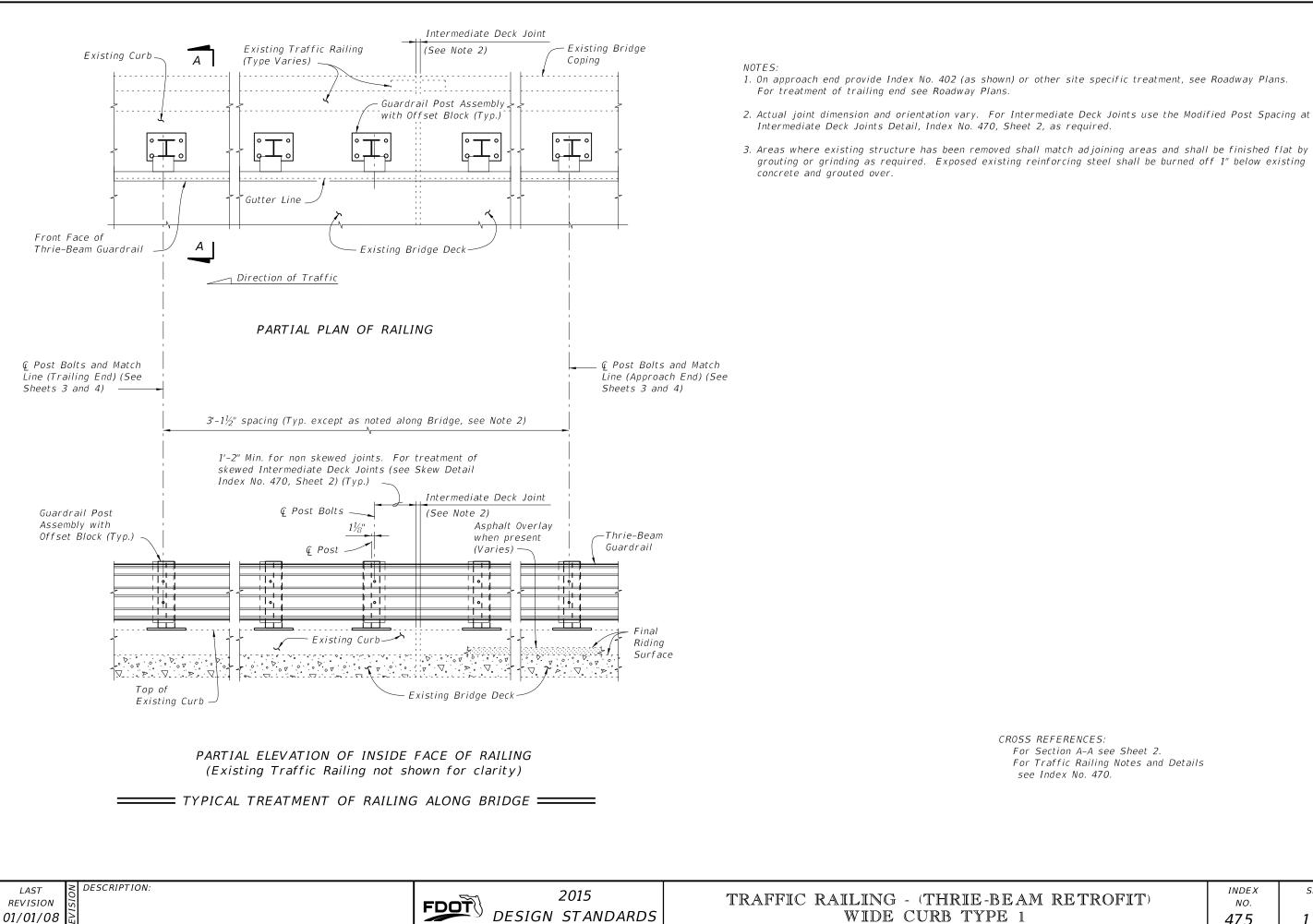
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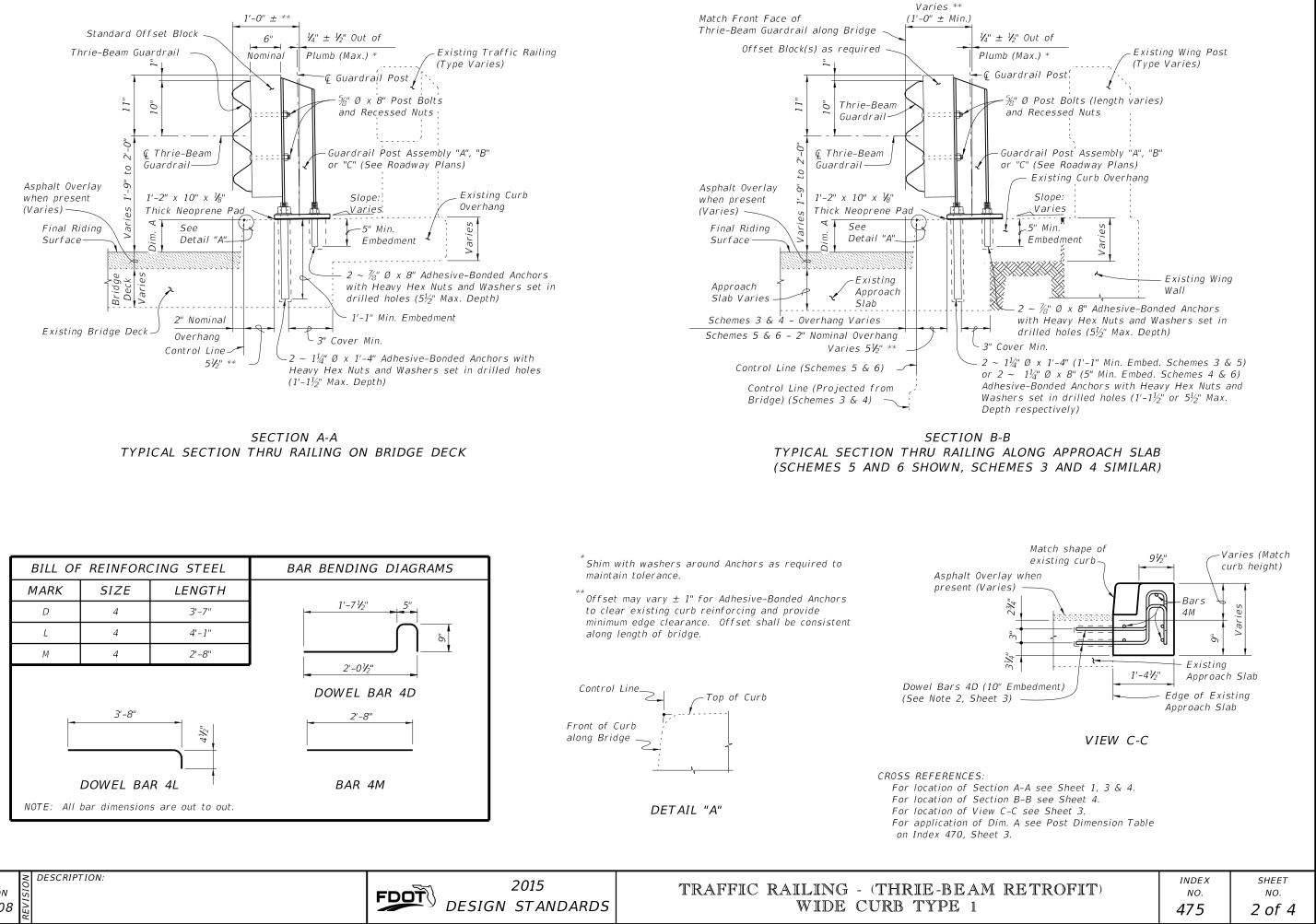


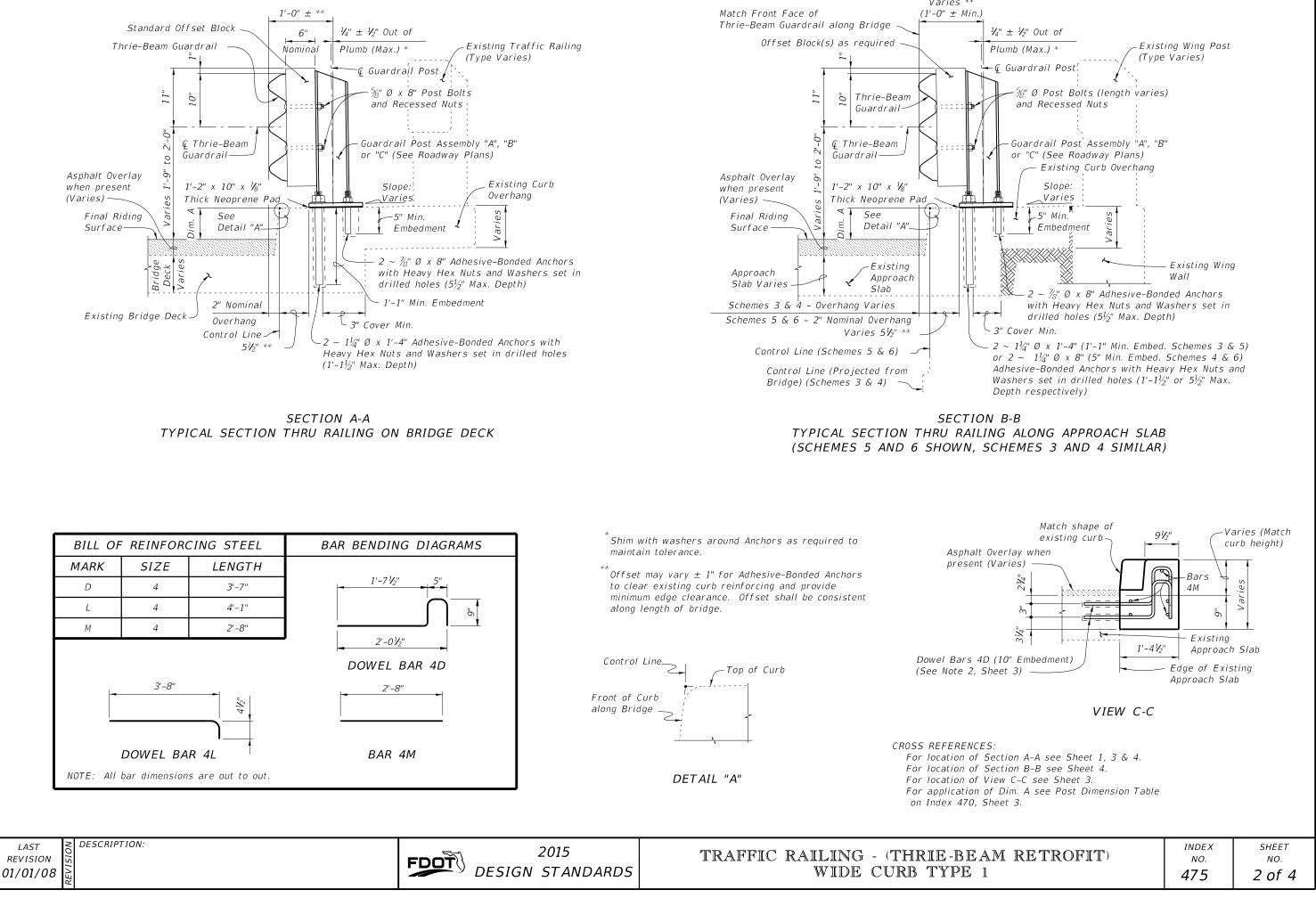


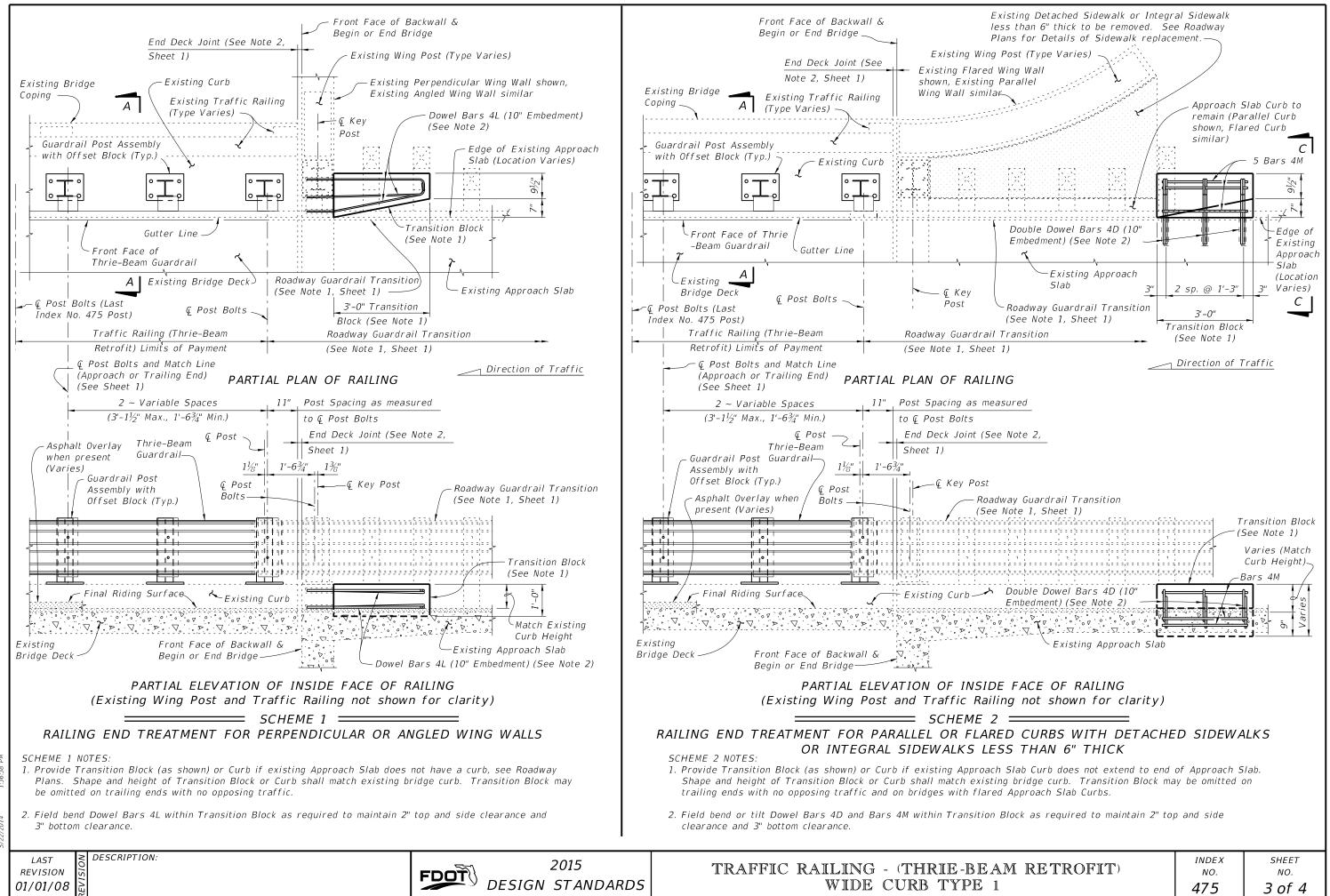
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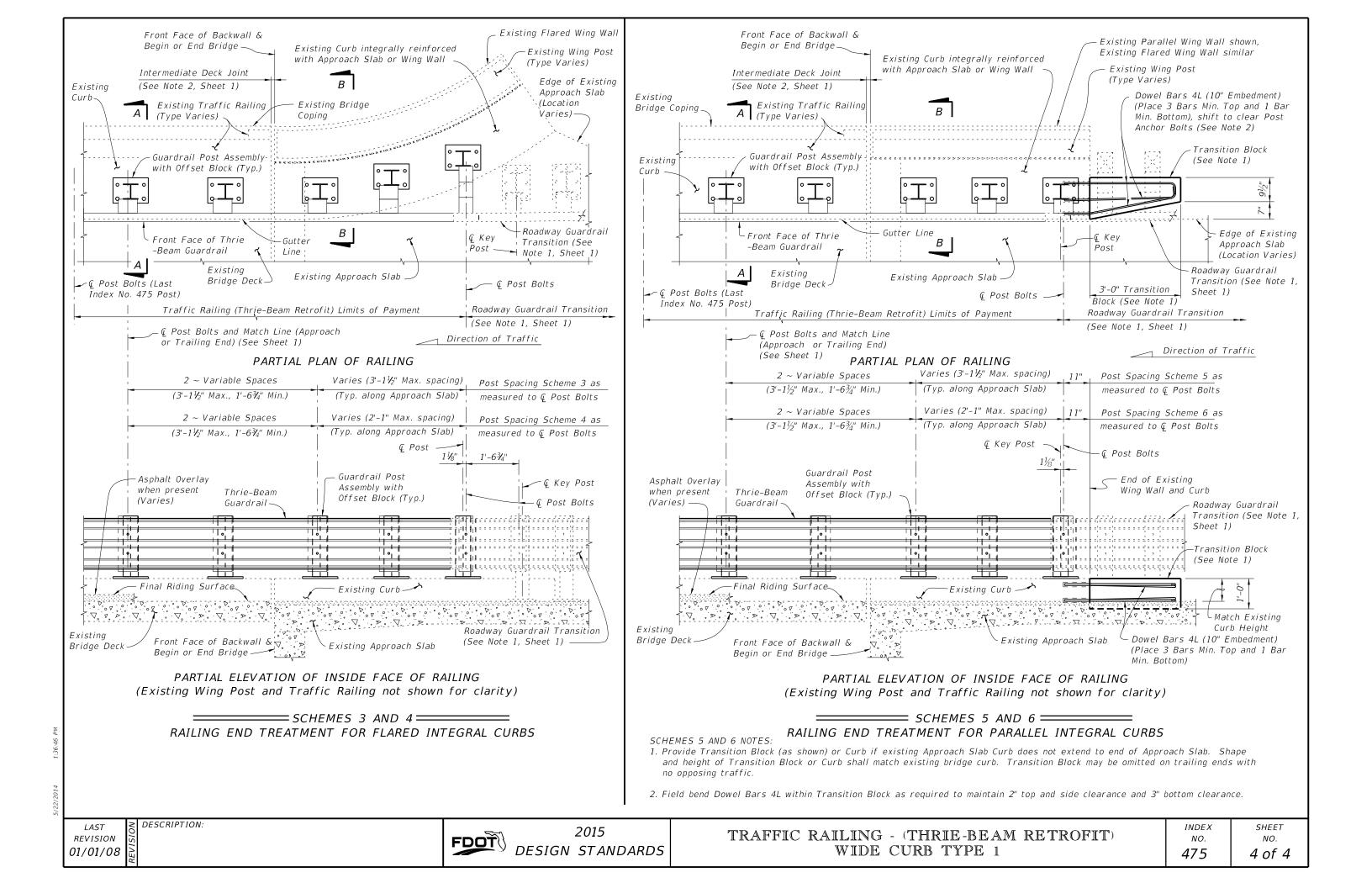
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

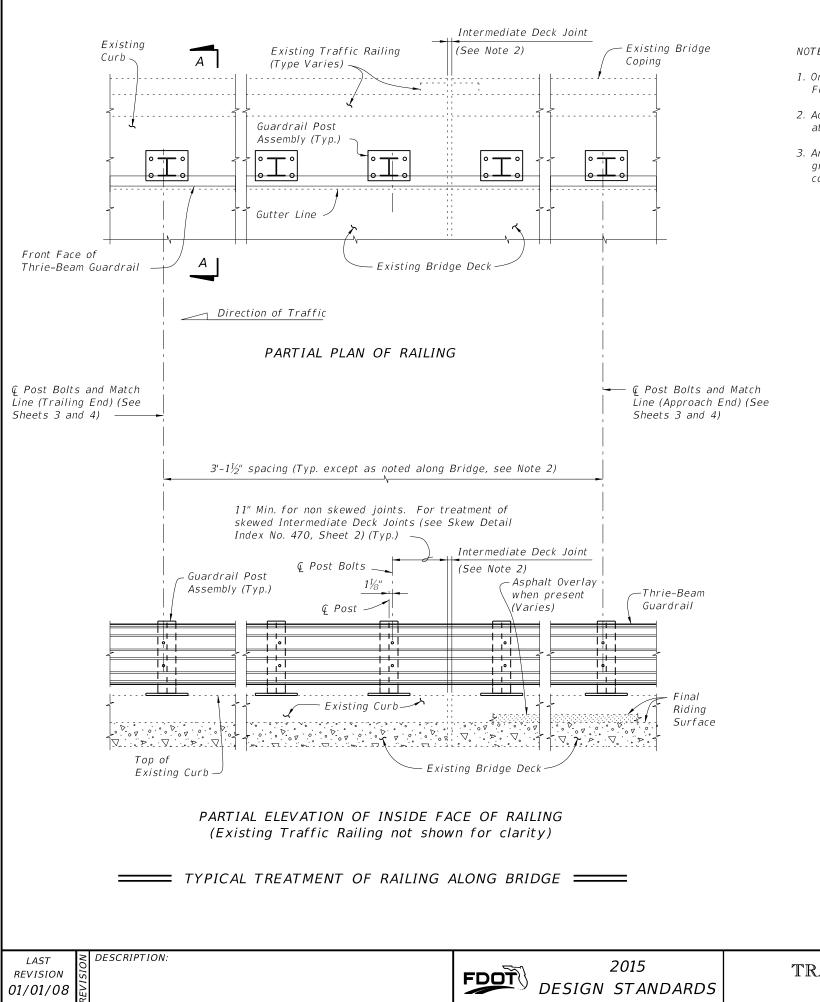
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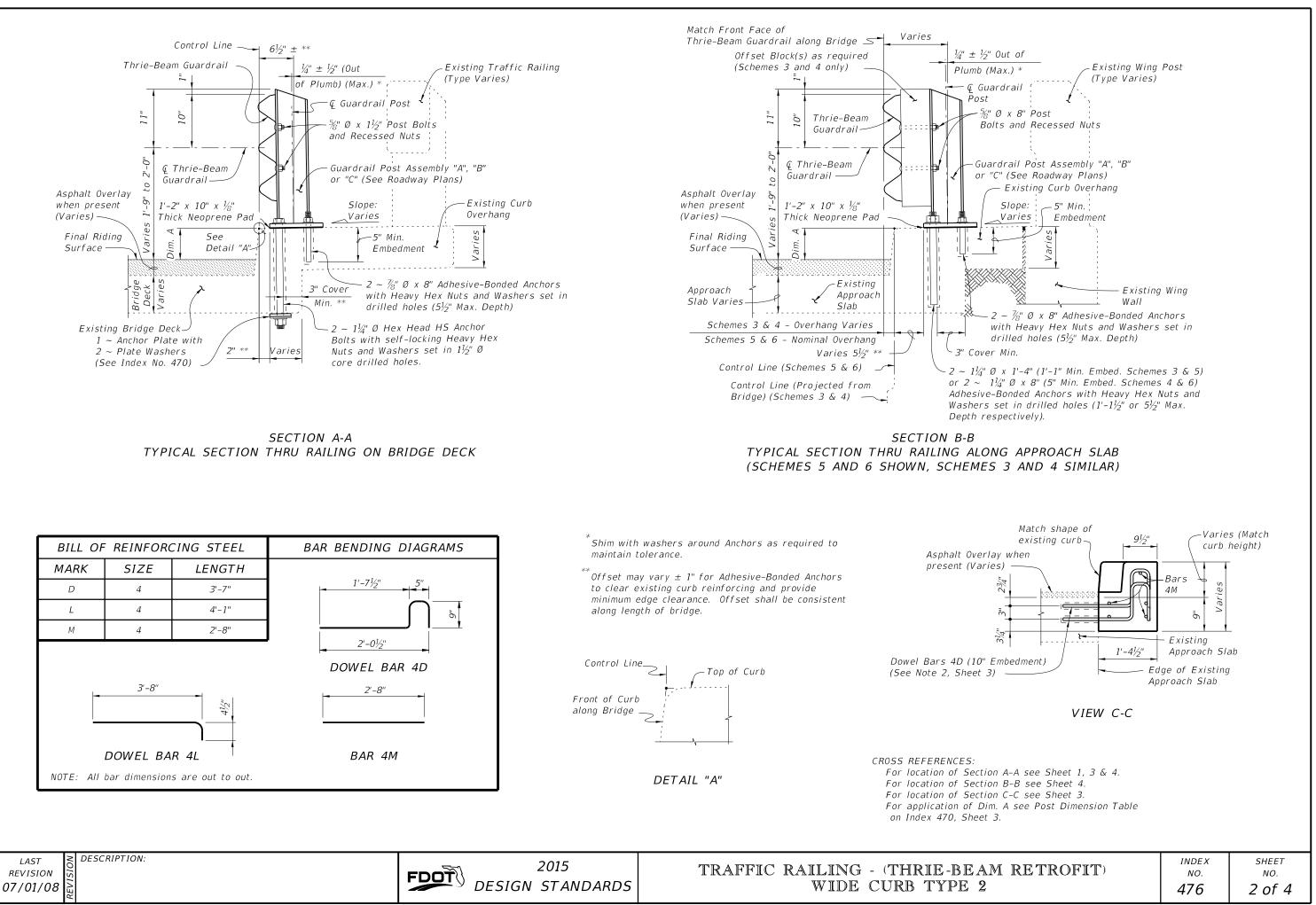


# NOTES:

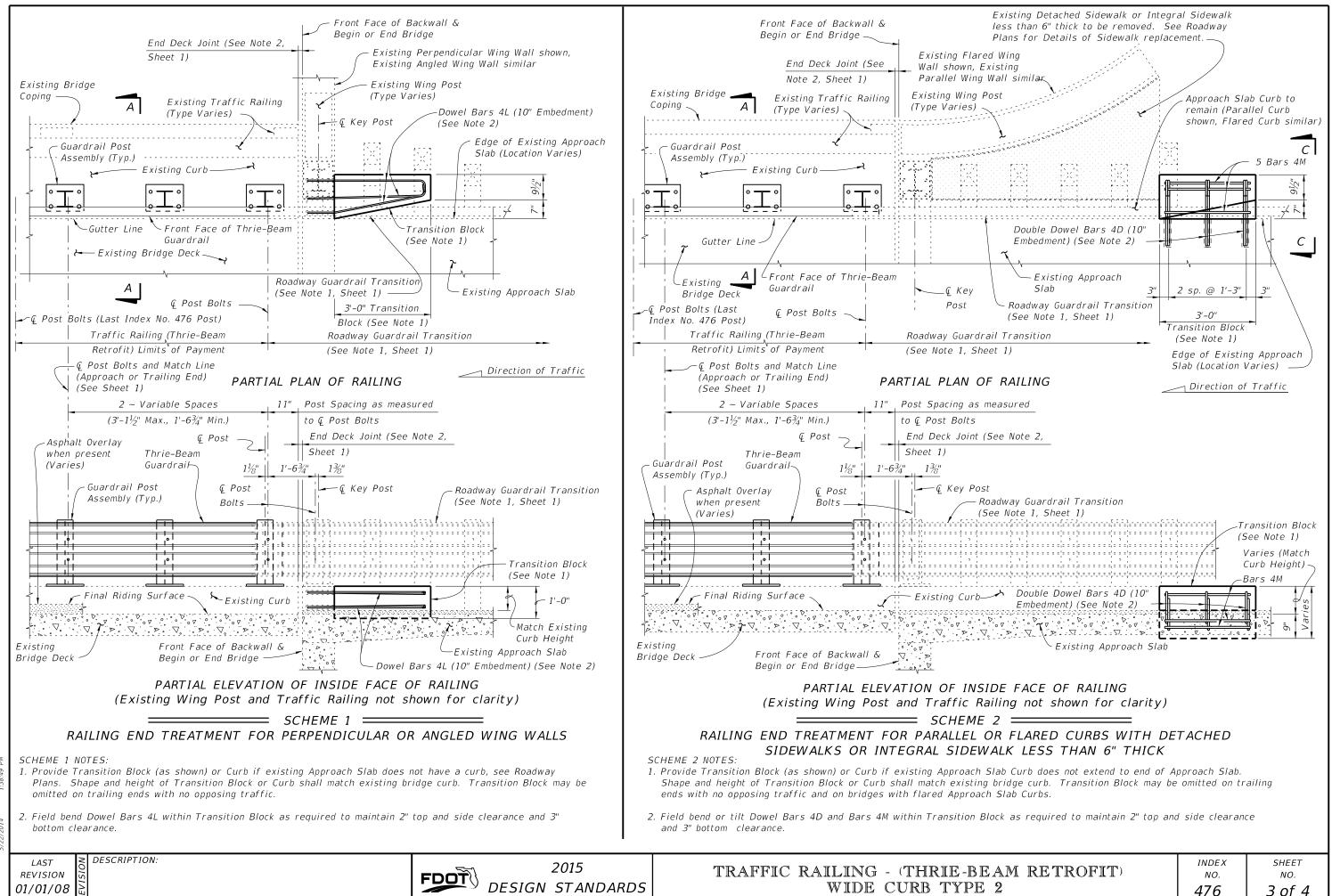
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

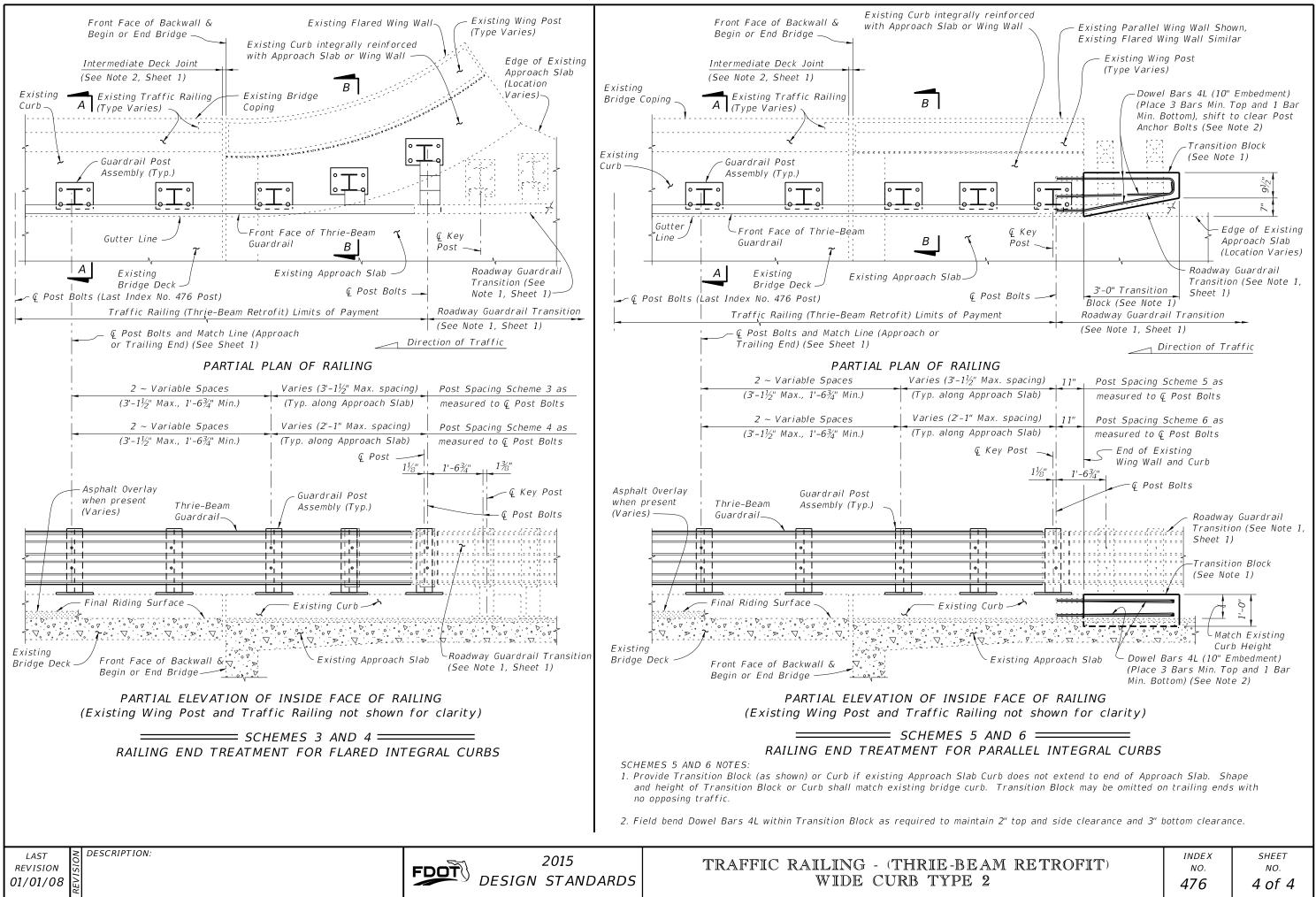
A RETROFIT	INDEX NO.	SHEET NO.
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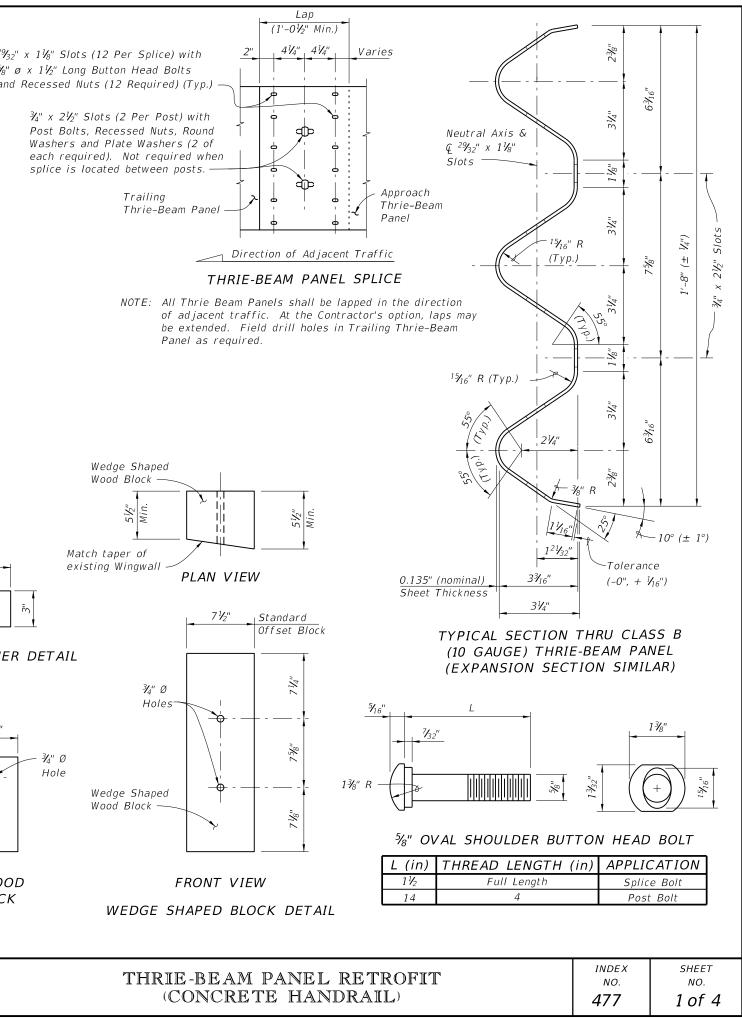
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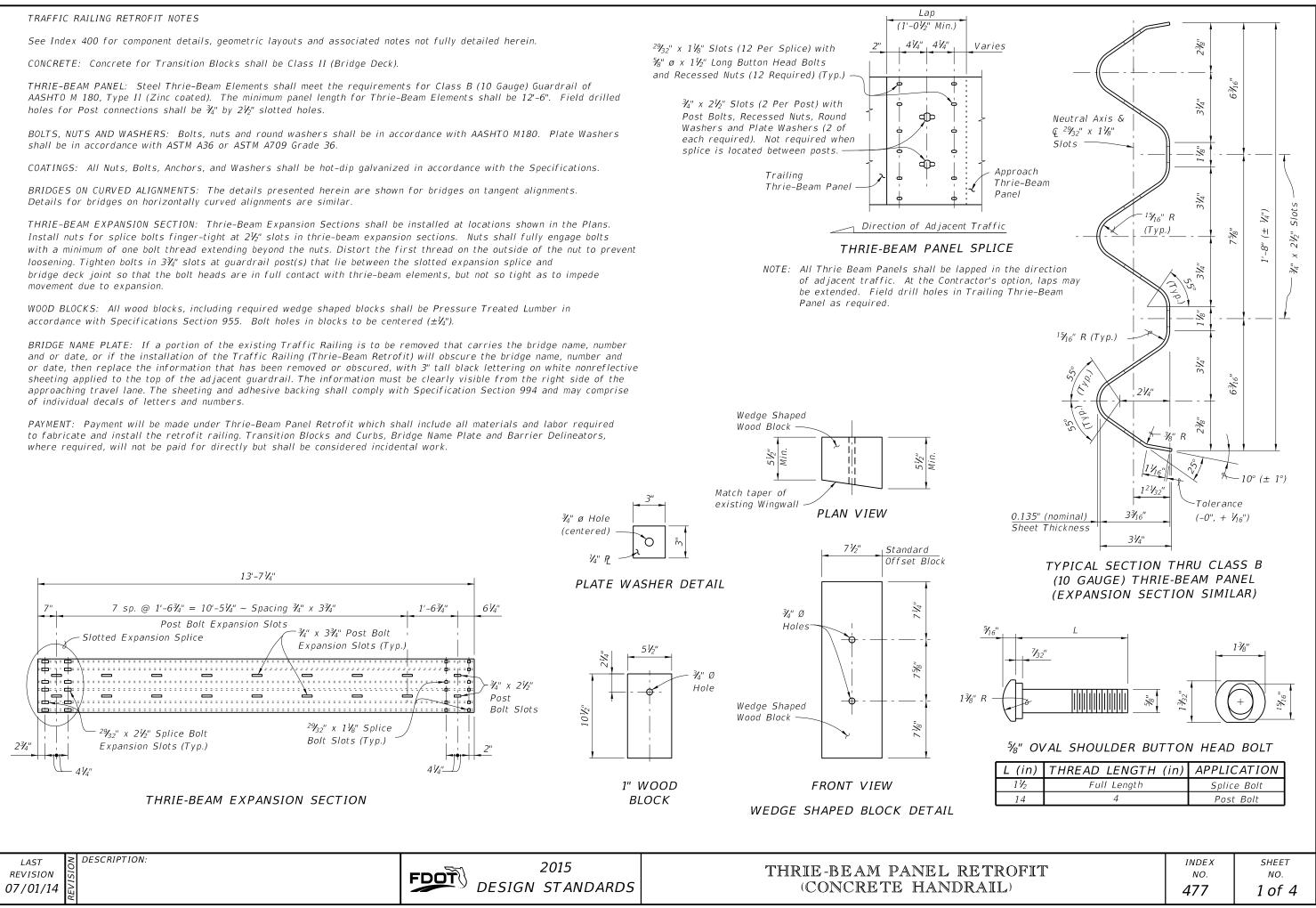


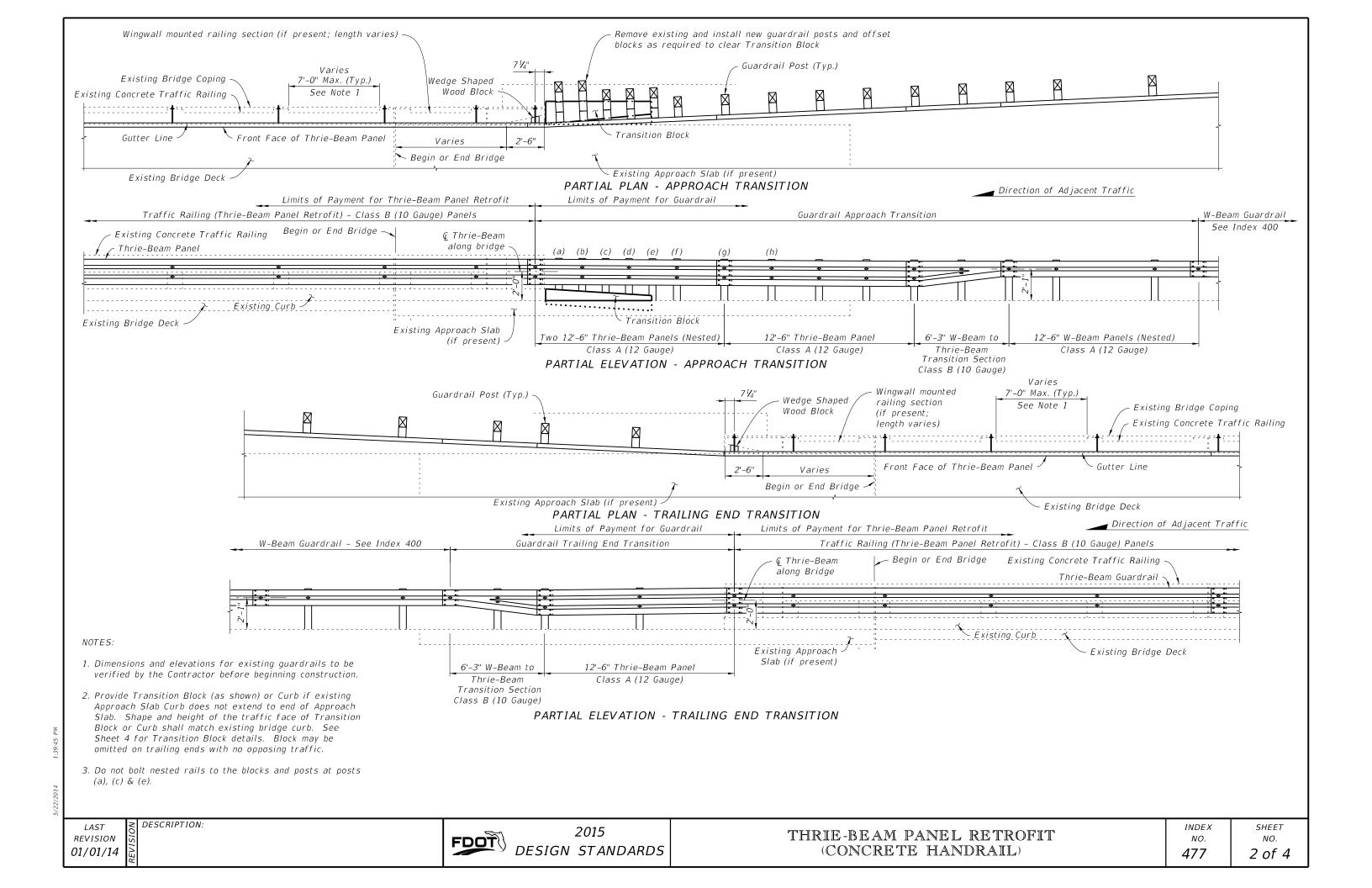
Details for bridges on horizontally curved alignments are similar.

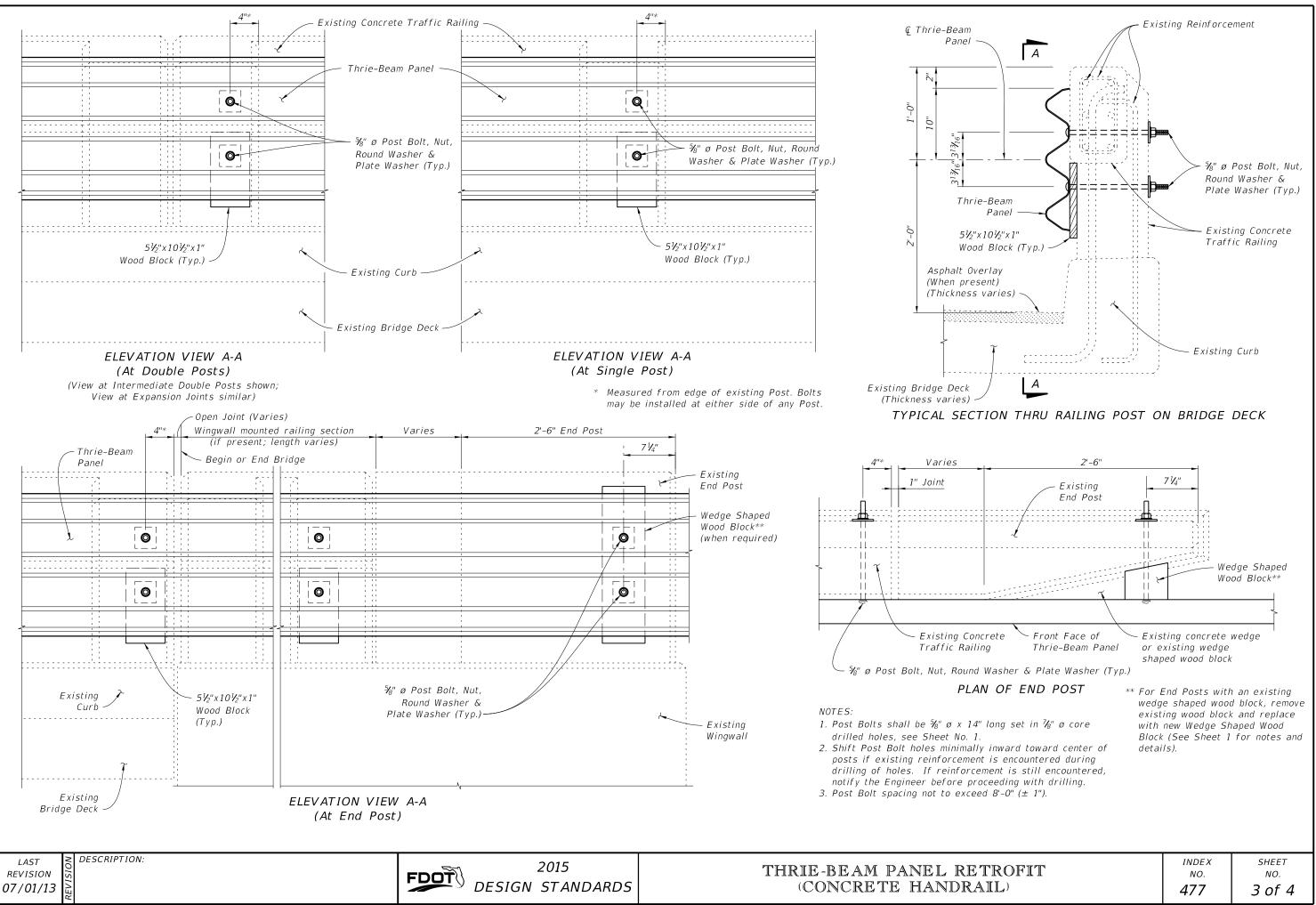
with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent movement due to expansion.

approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise

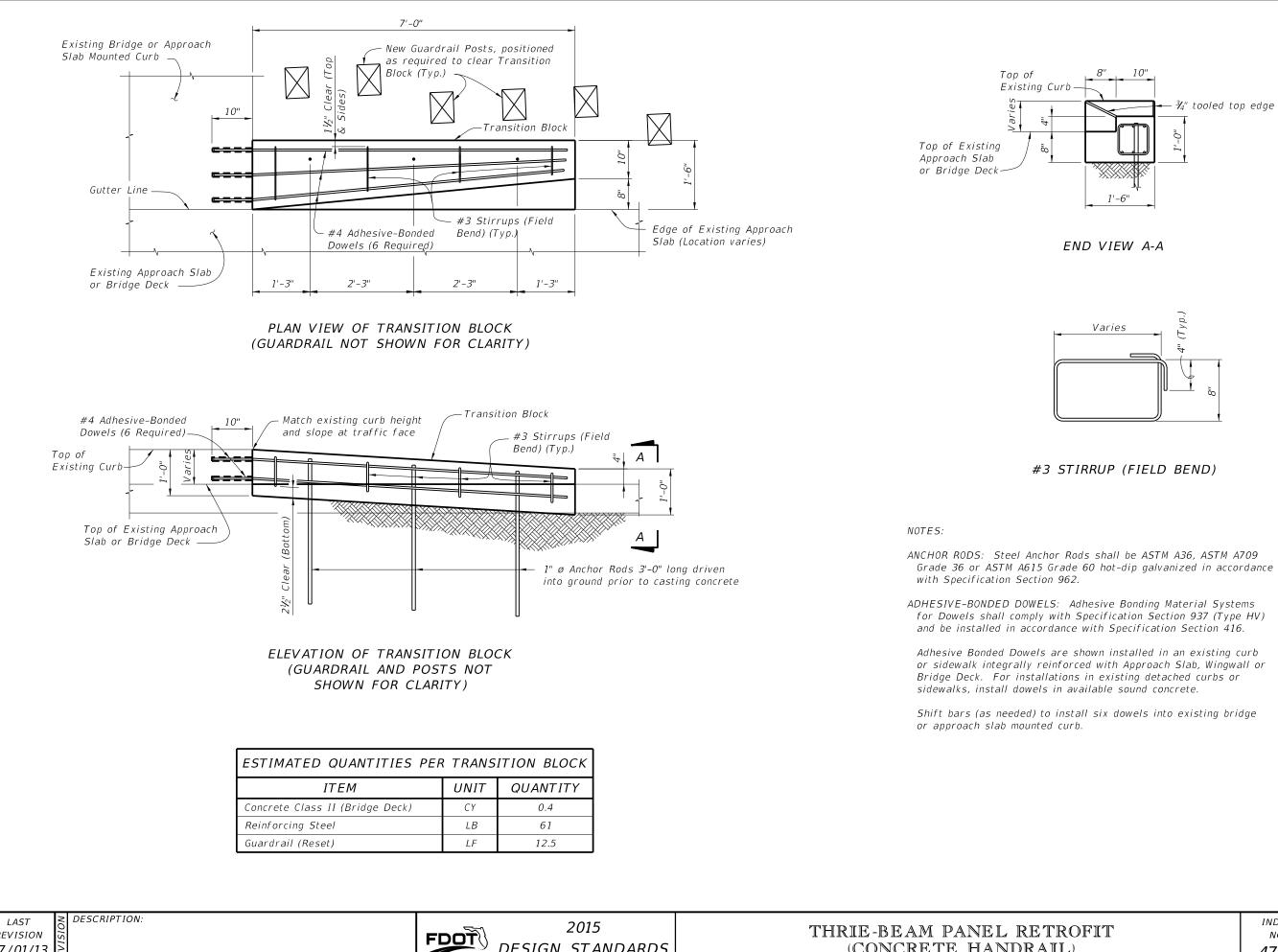








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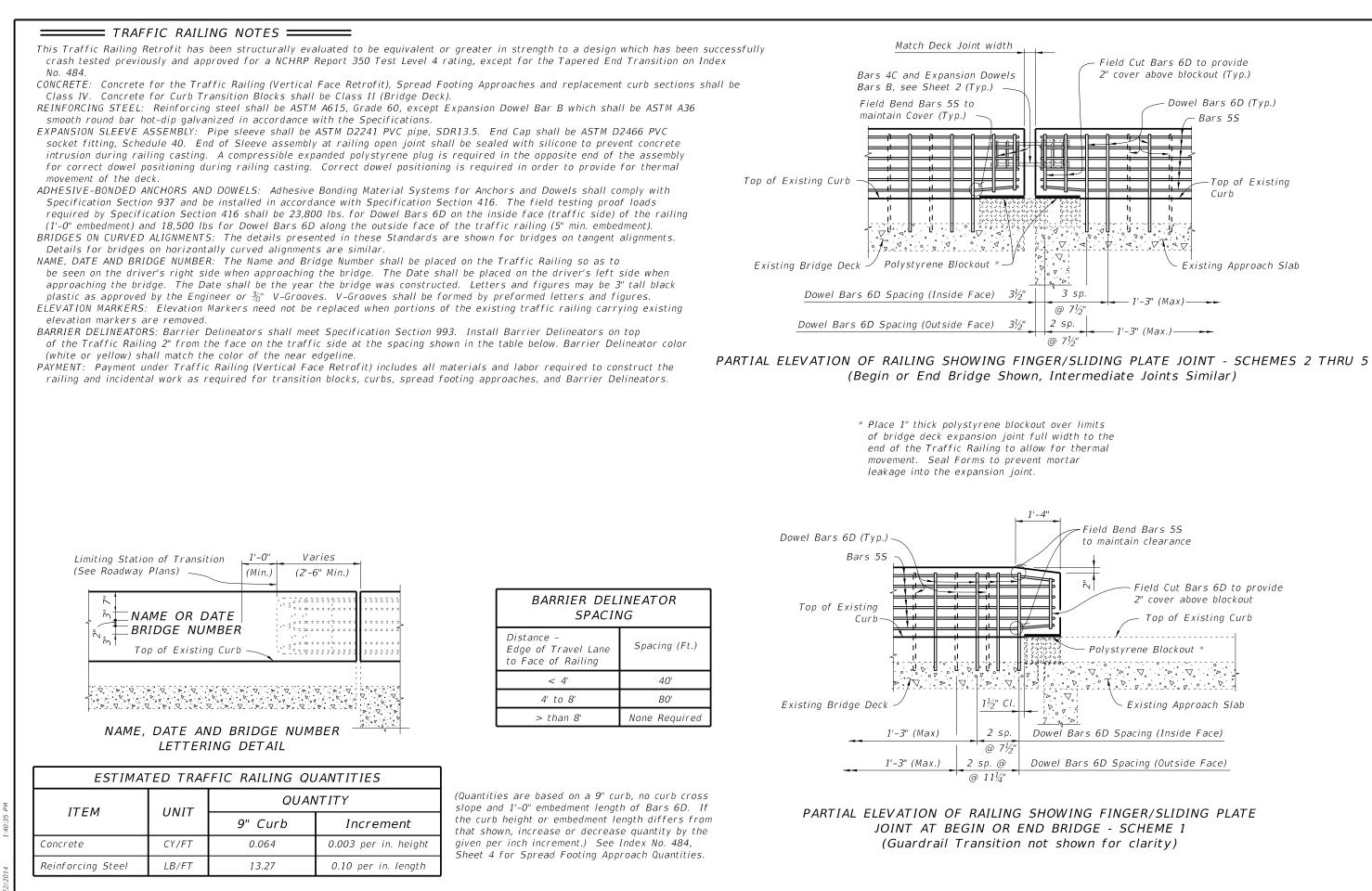


REVISION 07/01/13

DESIGN STANDARDS

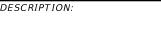
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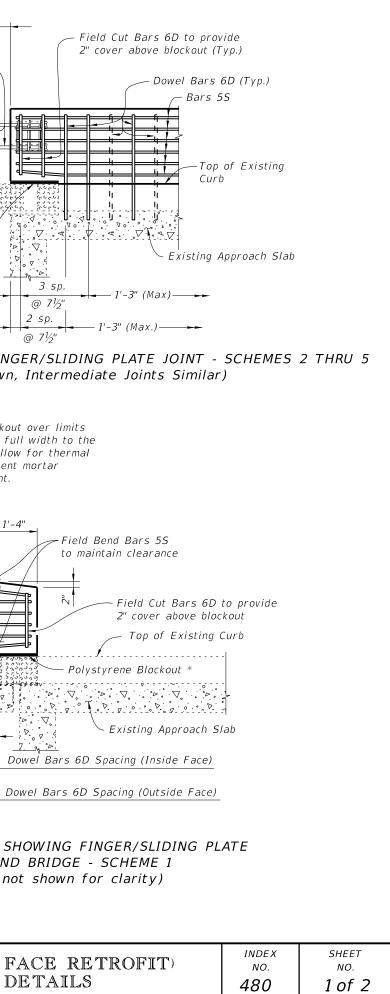
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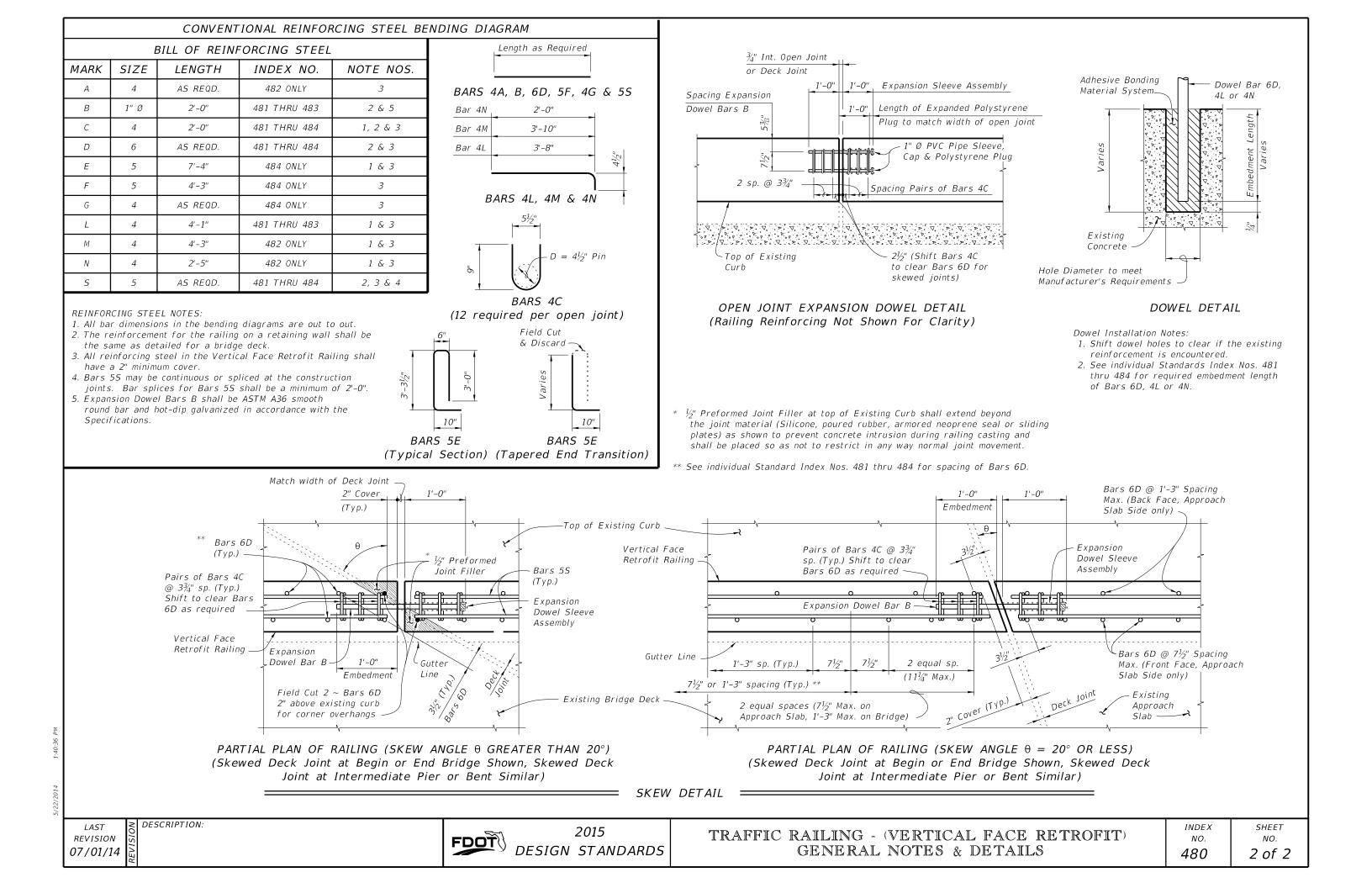
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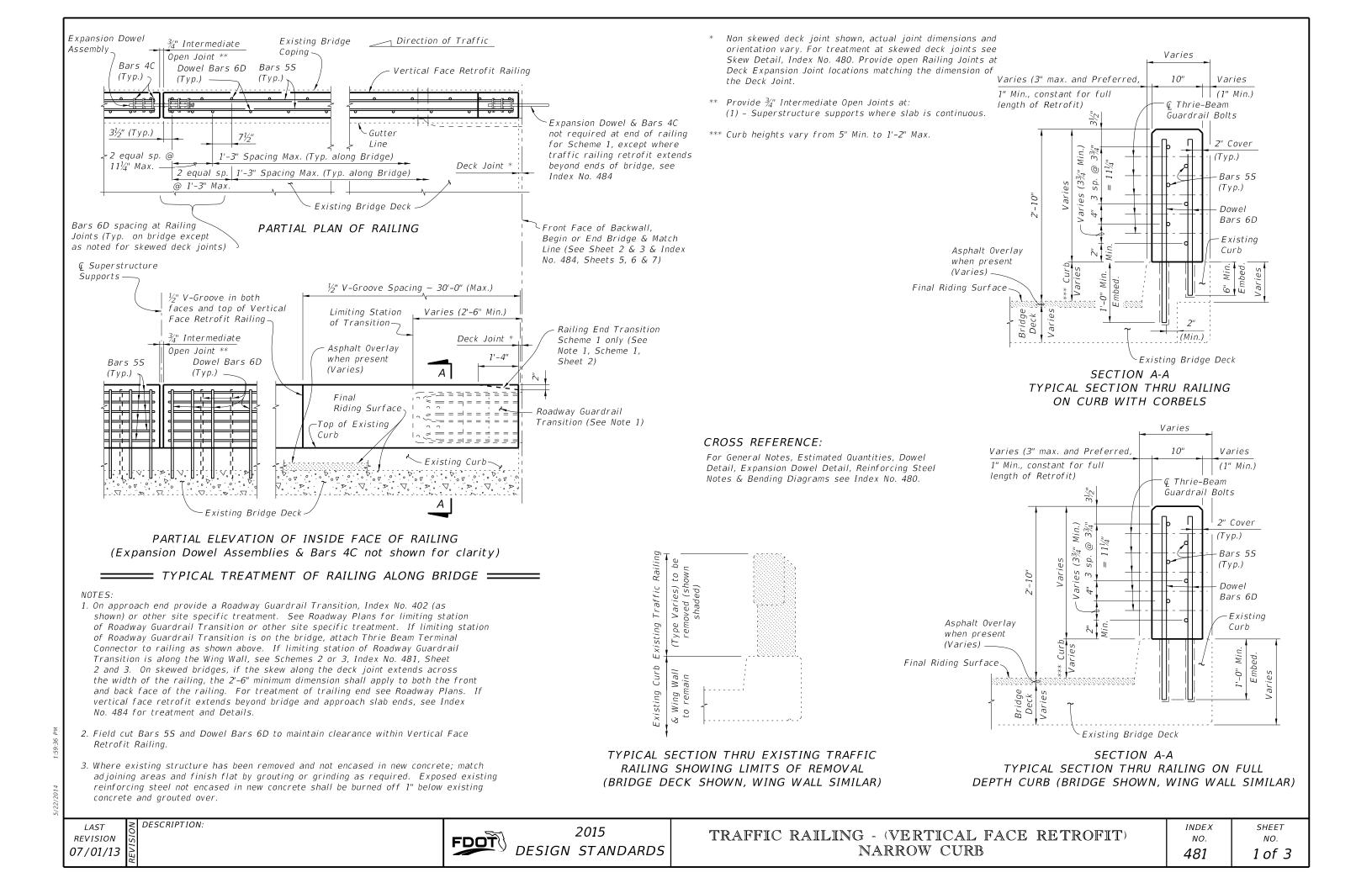


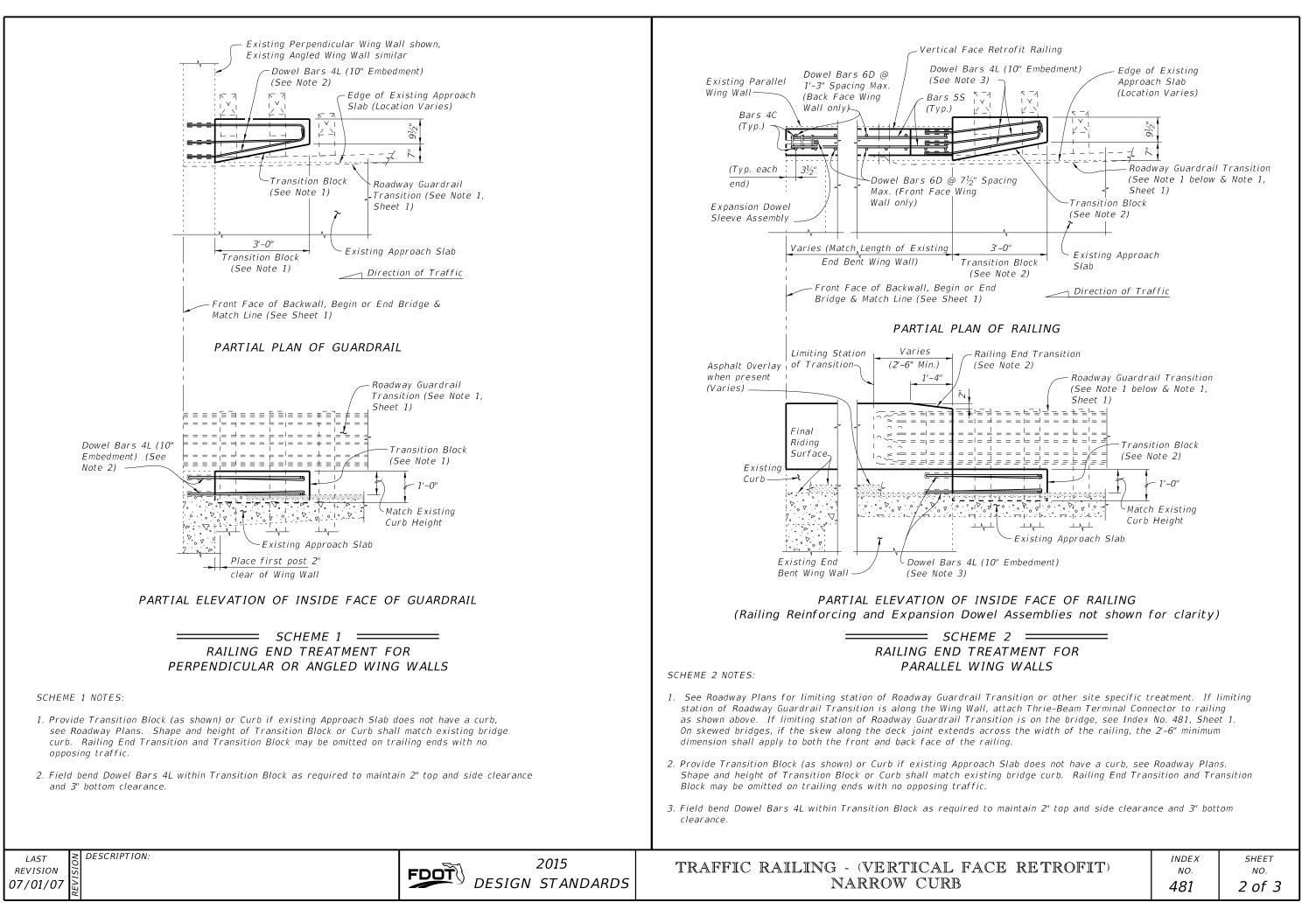


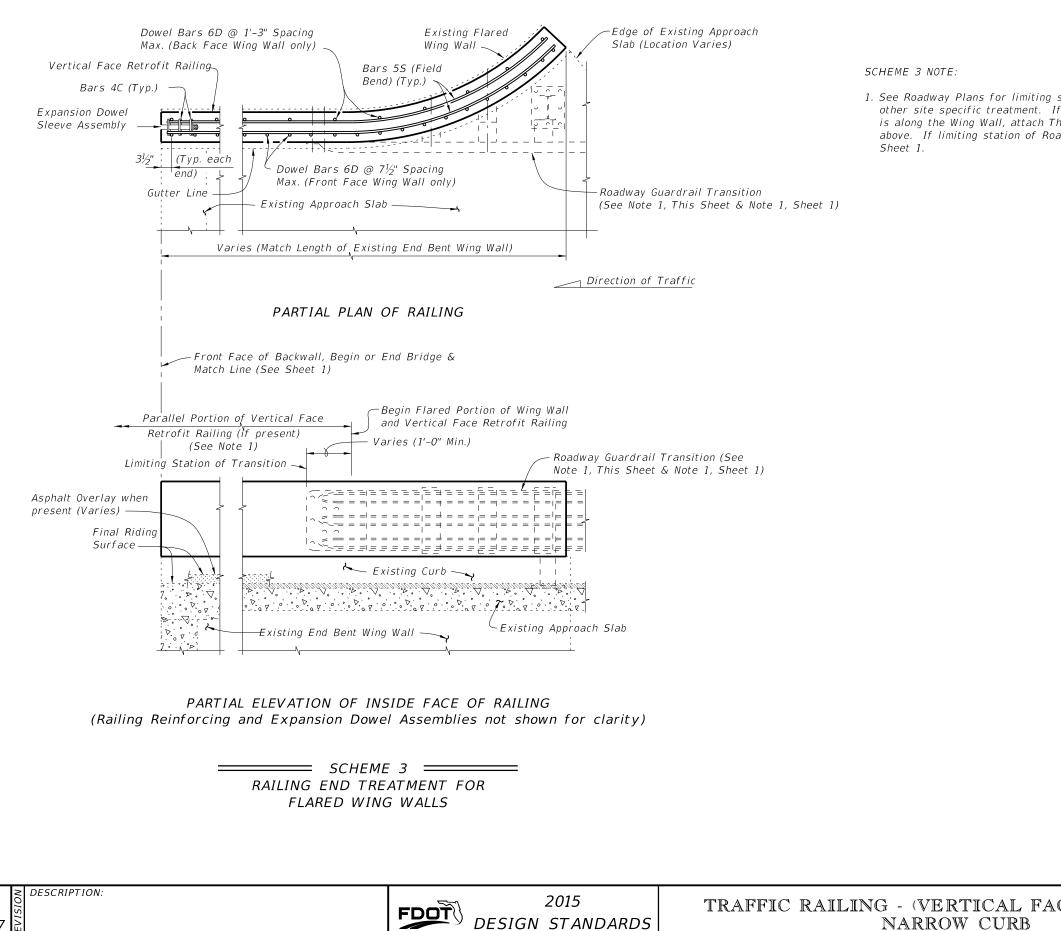
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) GENERAL NOTES AND DETAILS







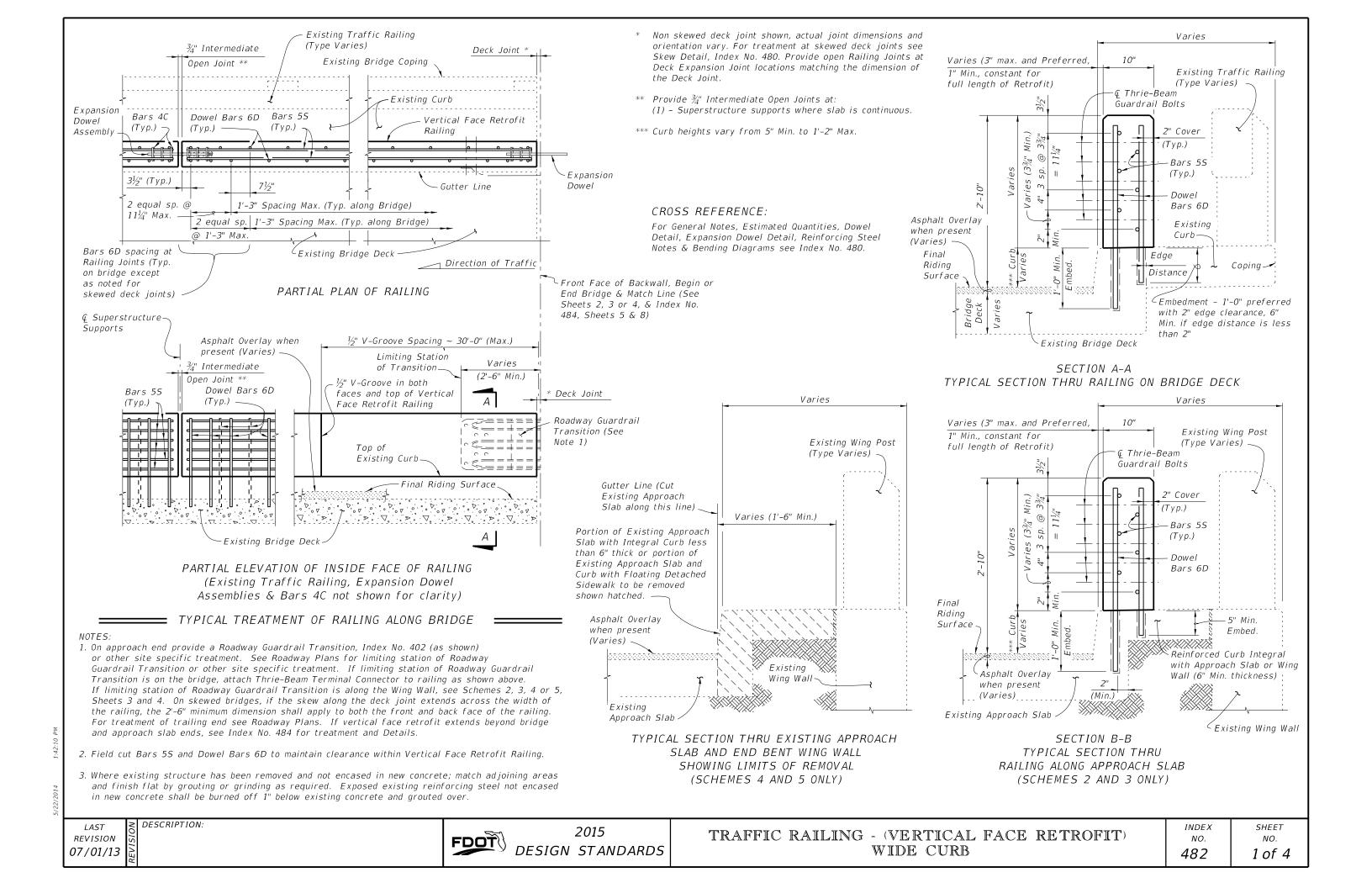


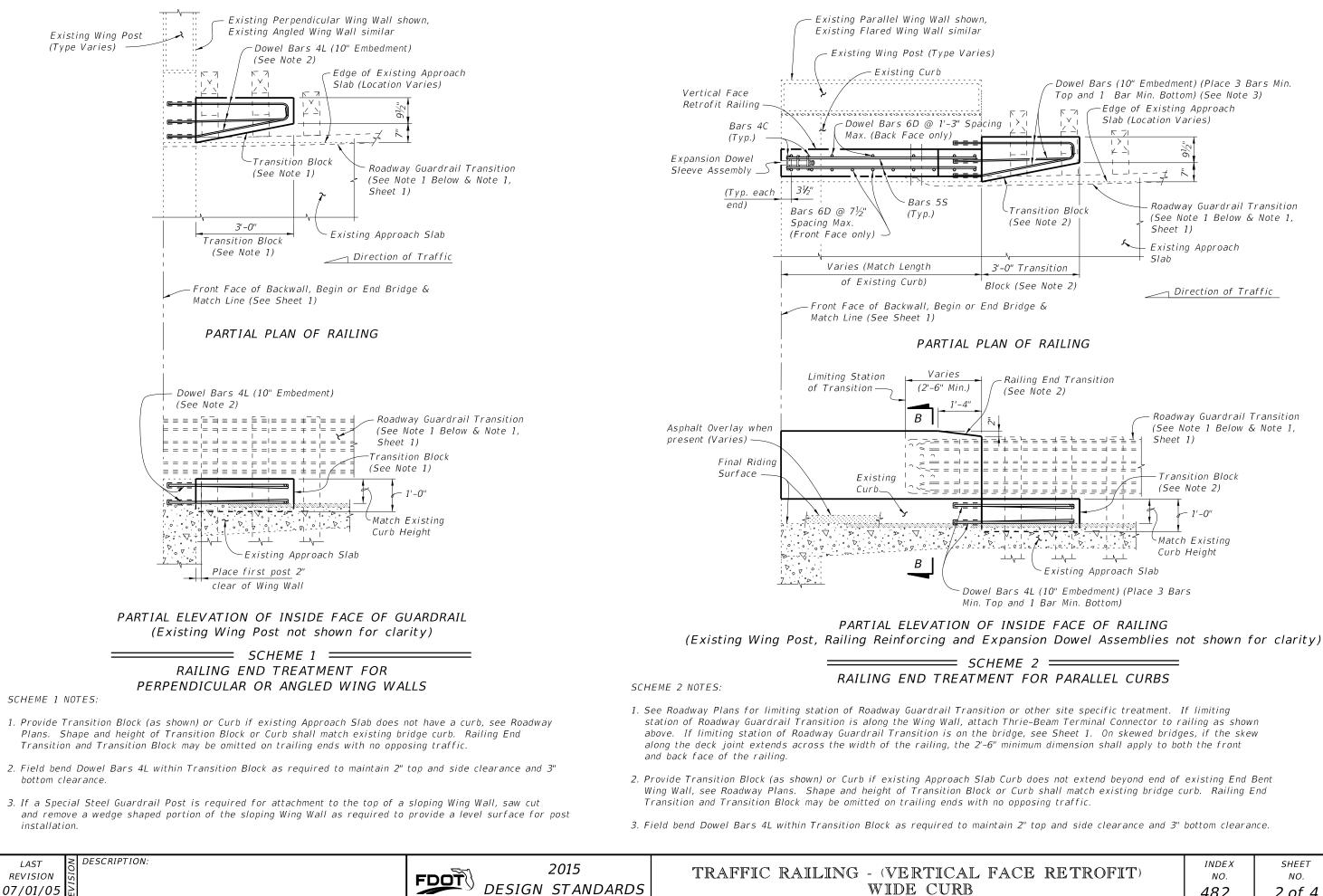


LAST REVISION 07/01/07

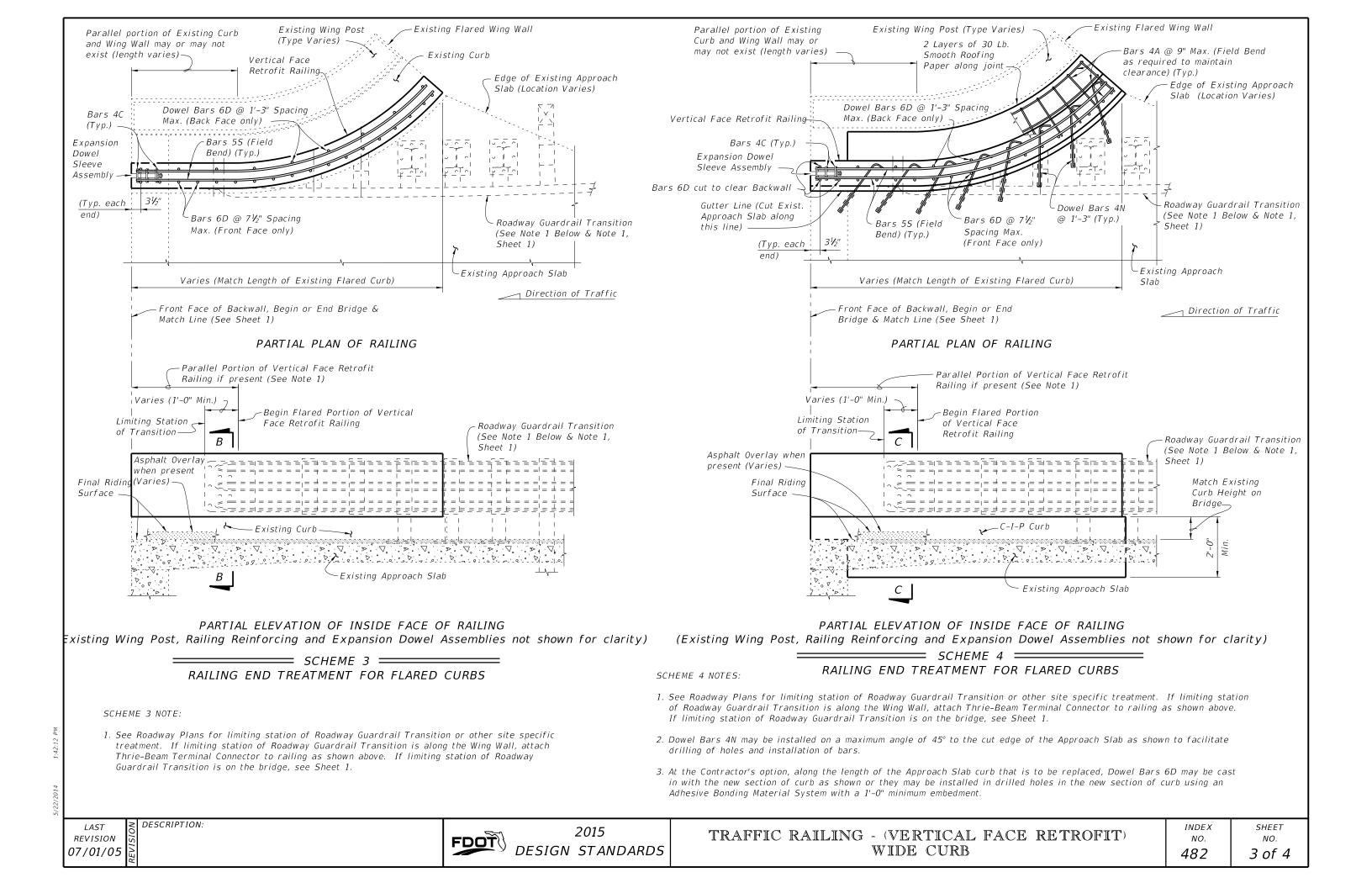
1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see

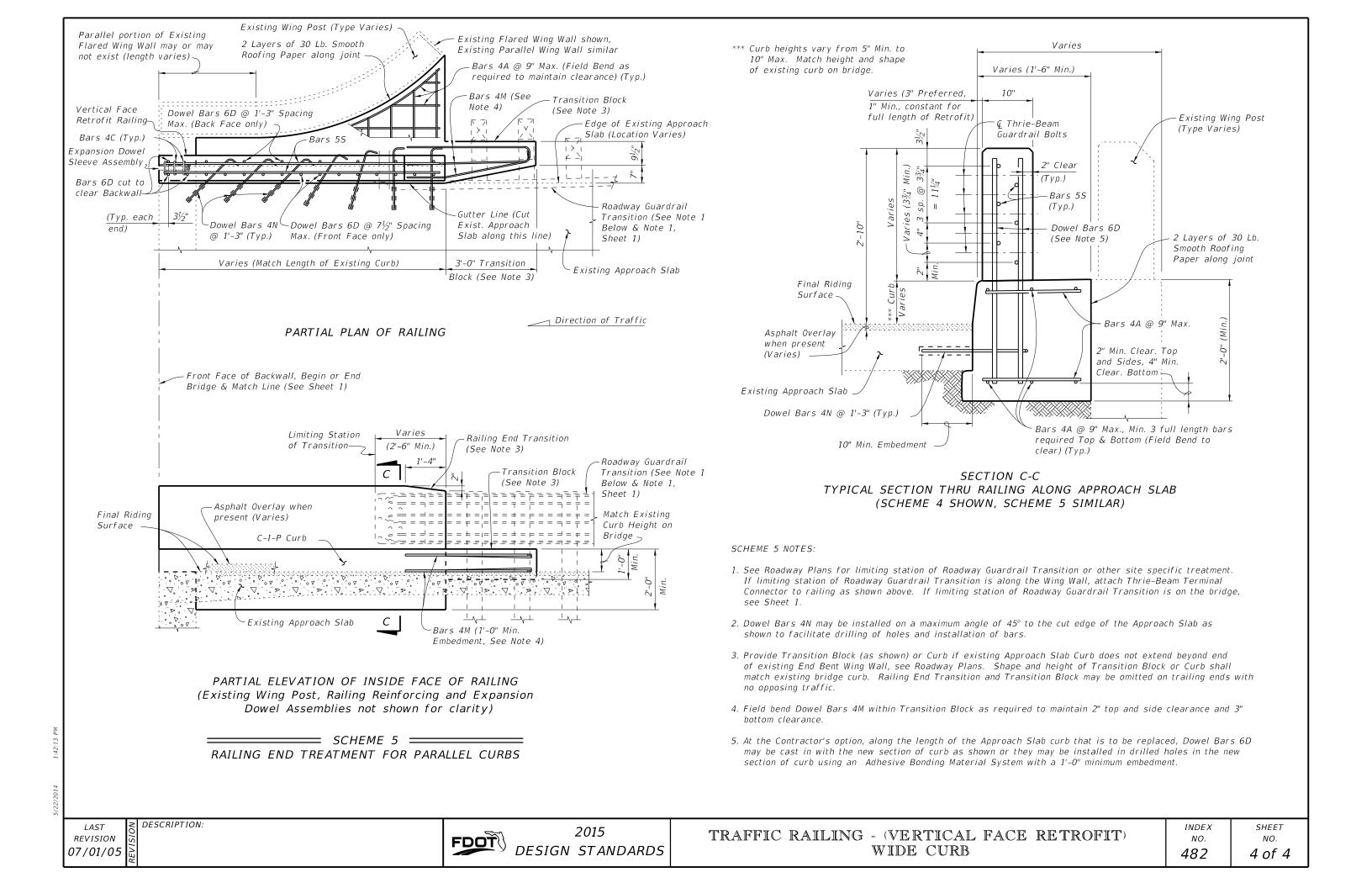
CE RETROFIT	INDEX NO.	SHEET NO.
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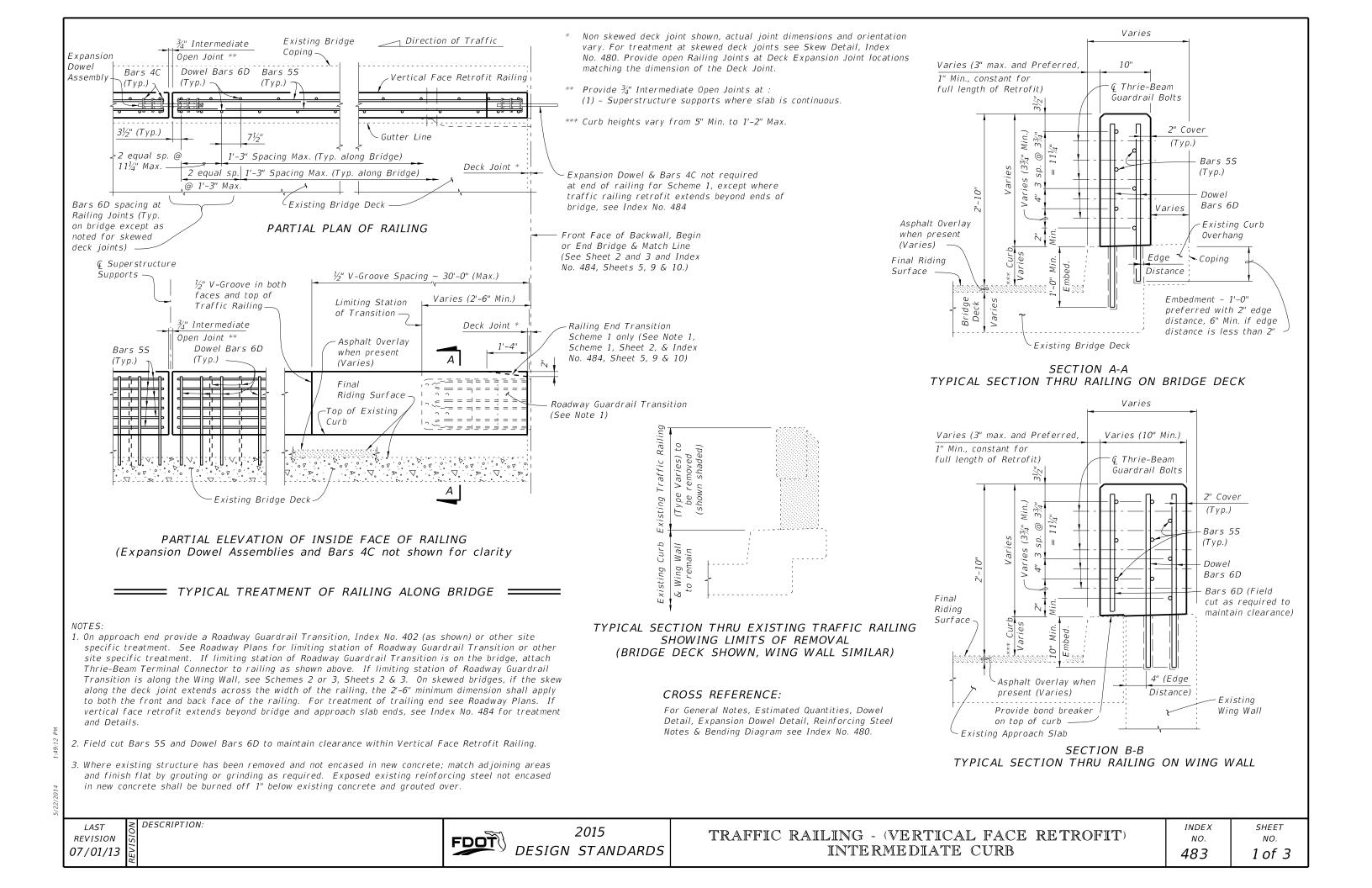


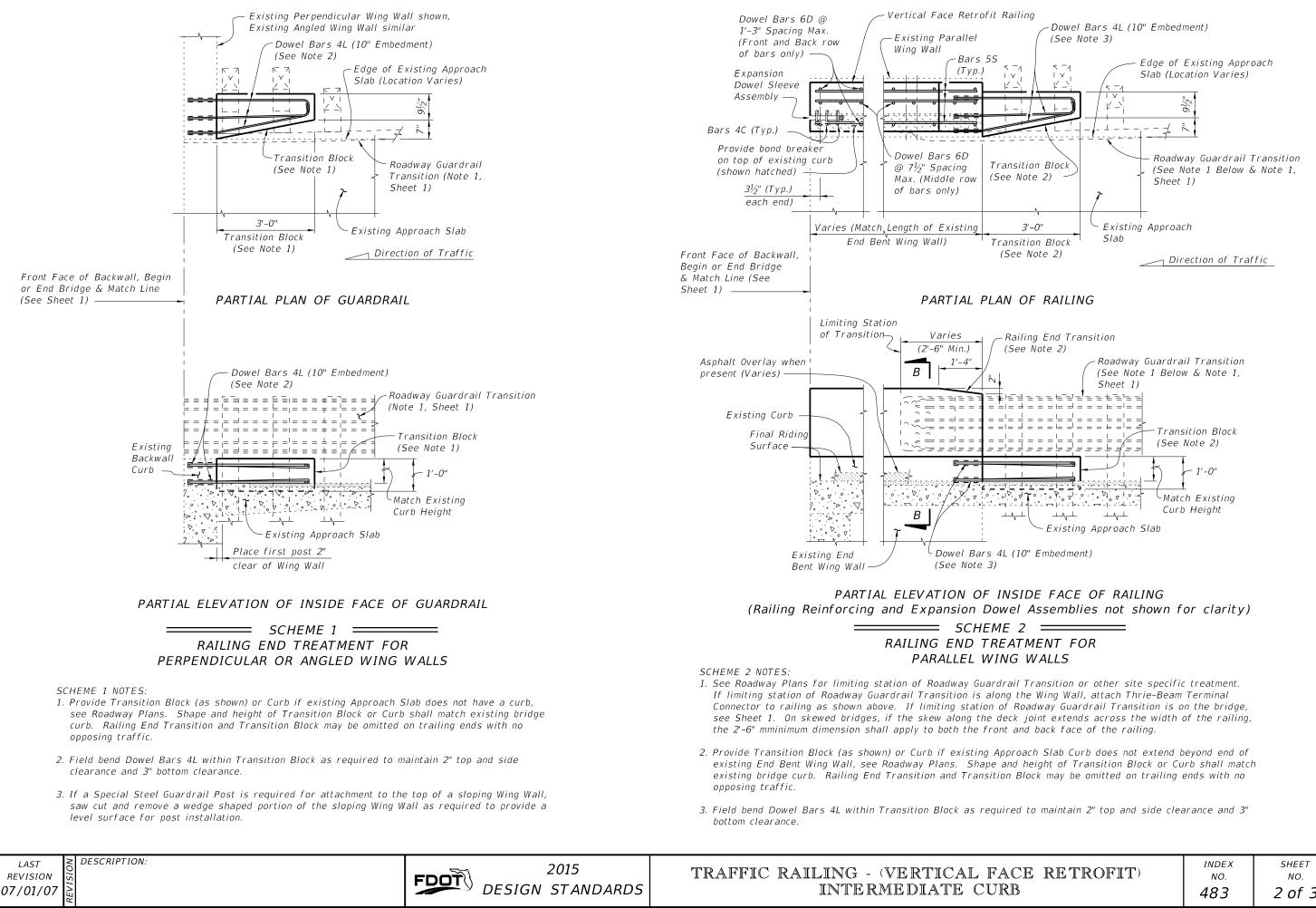


CE RETROFIT)	INDEX NO.	SHEET NO.
	482	2 of 4

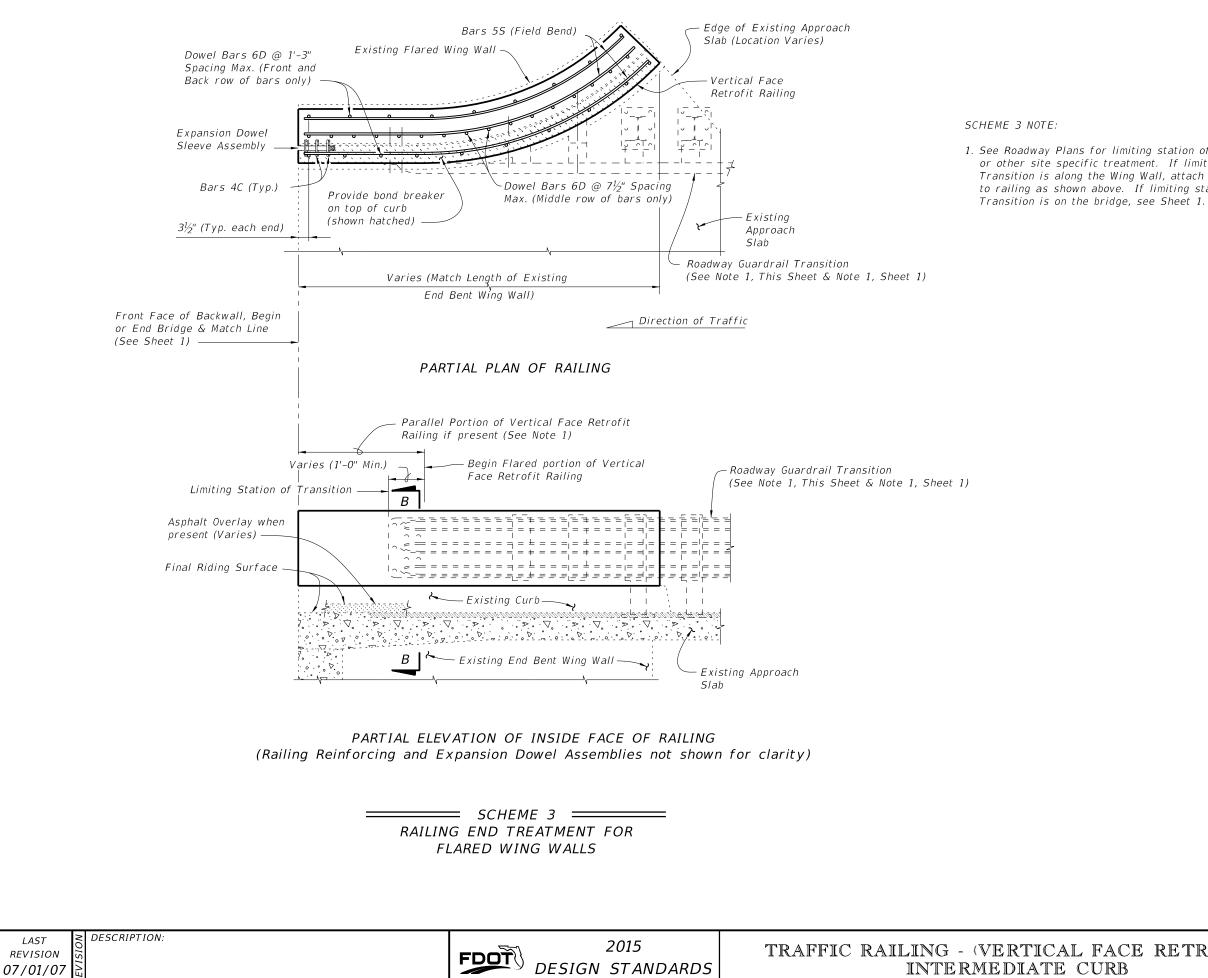






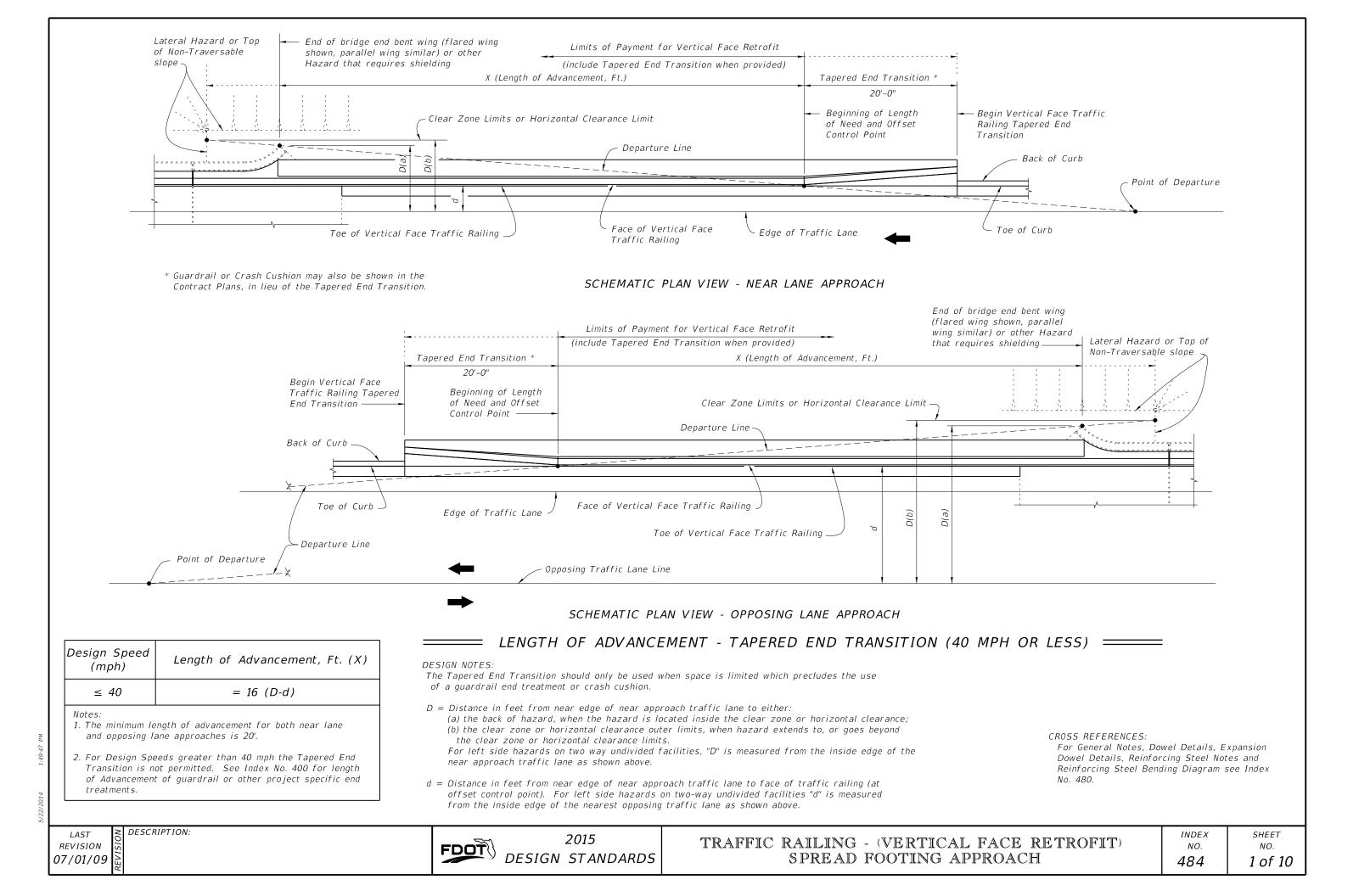


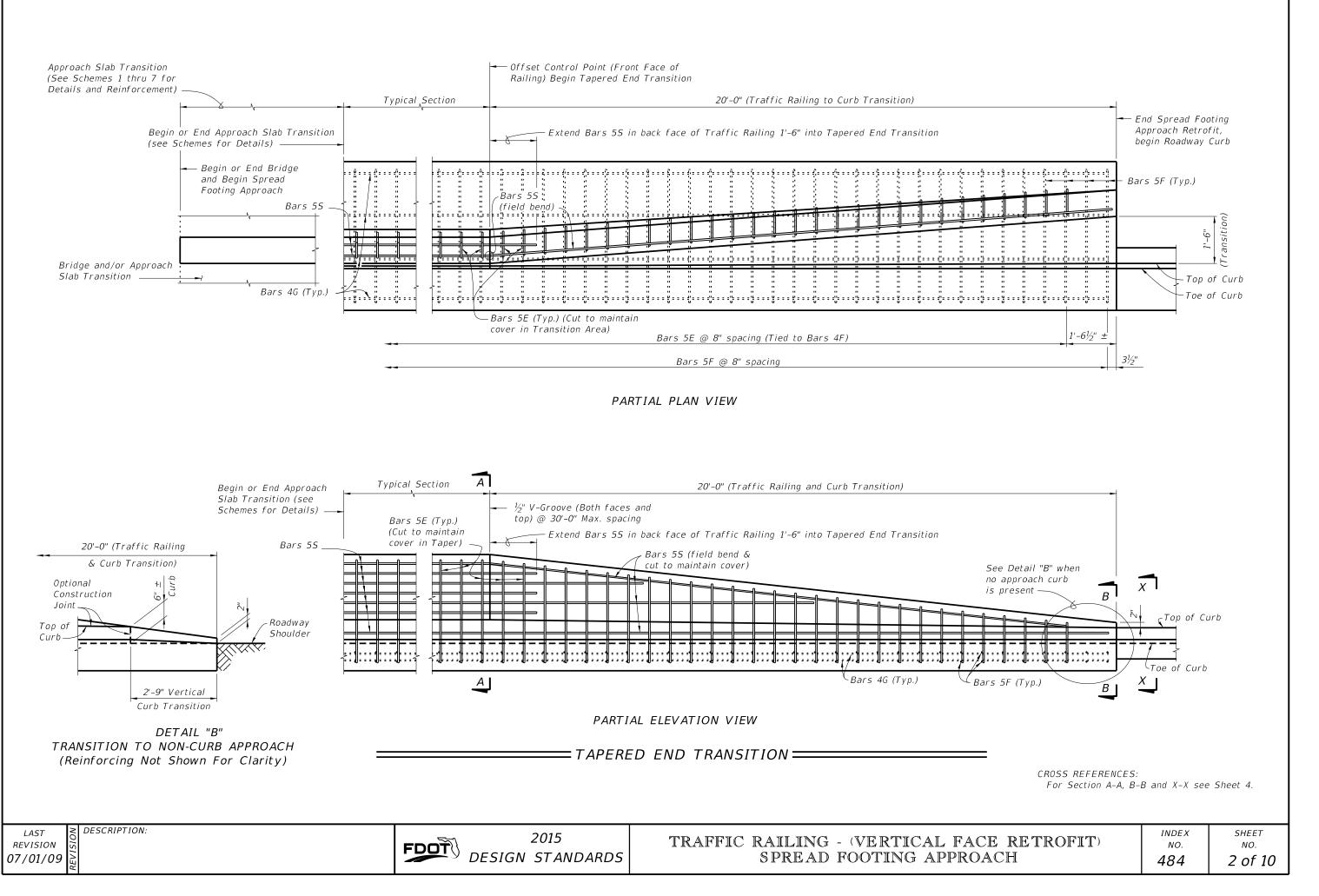
CE RETROFIT)	INDEX NO.	SHEET NO.
B	483	2 of 3

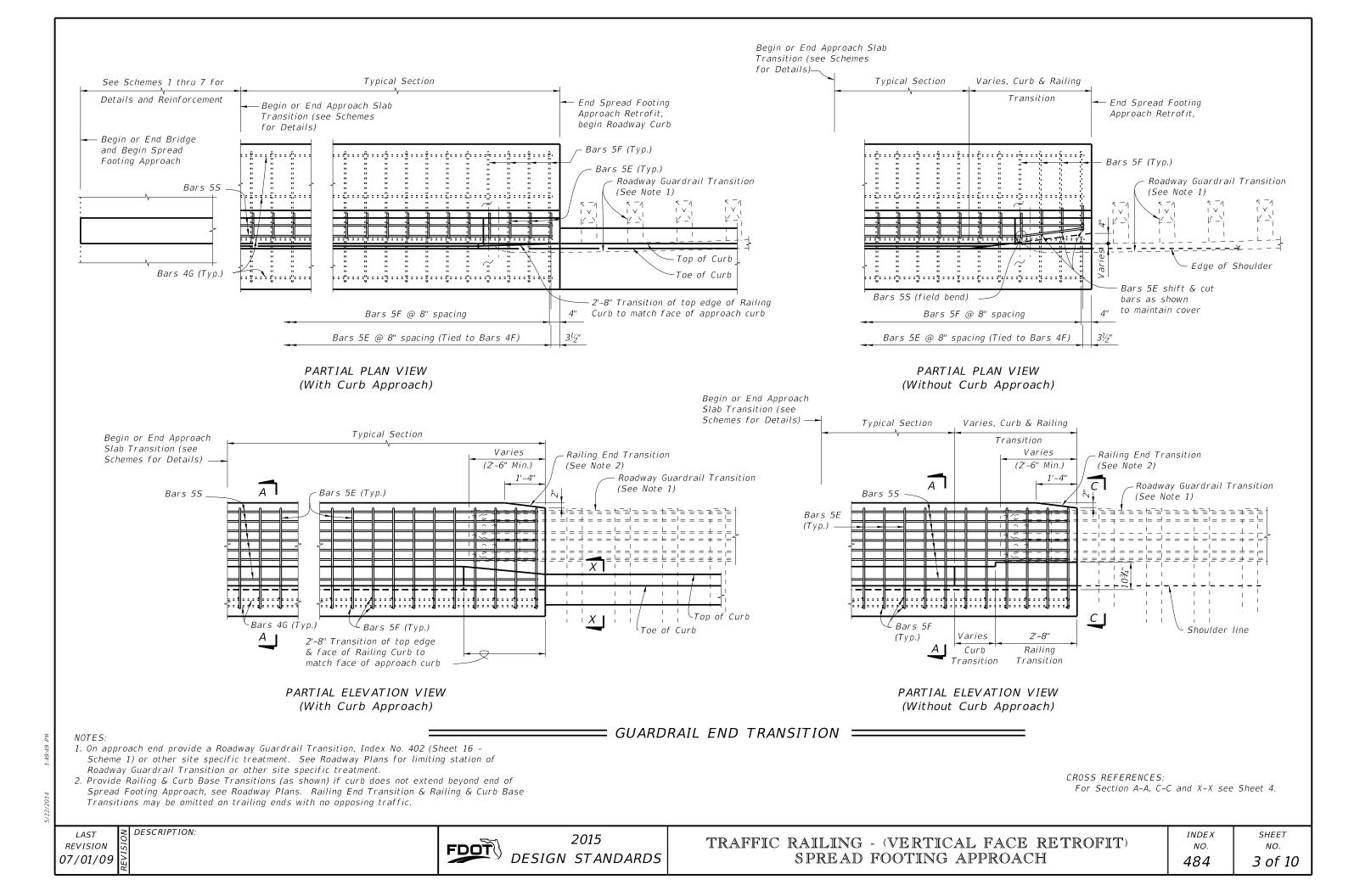


LAST REVISION 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail

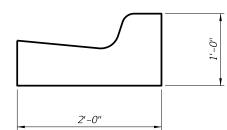
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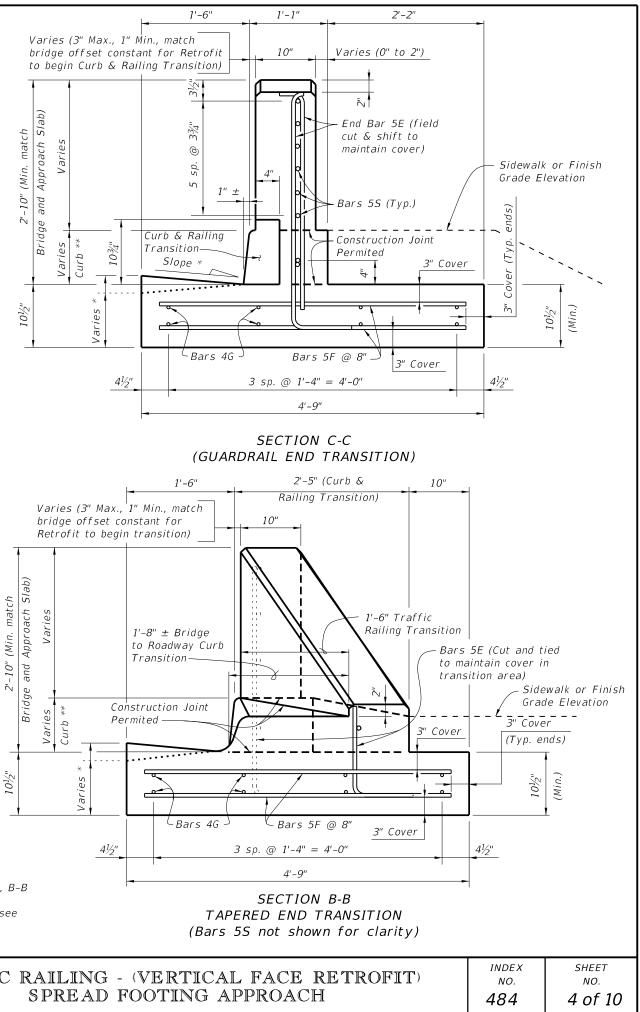




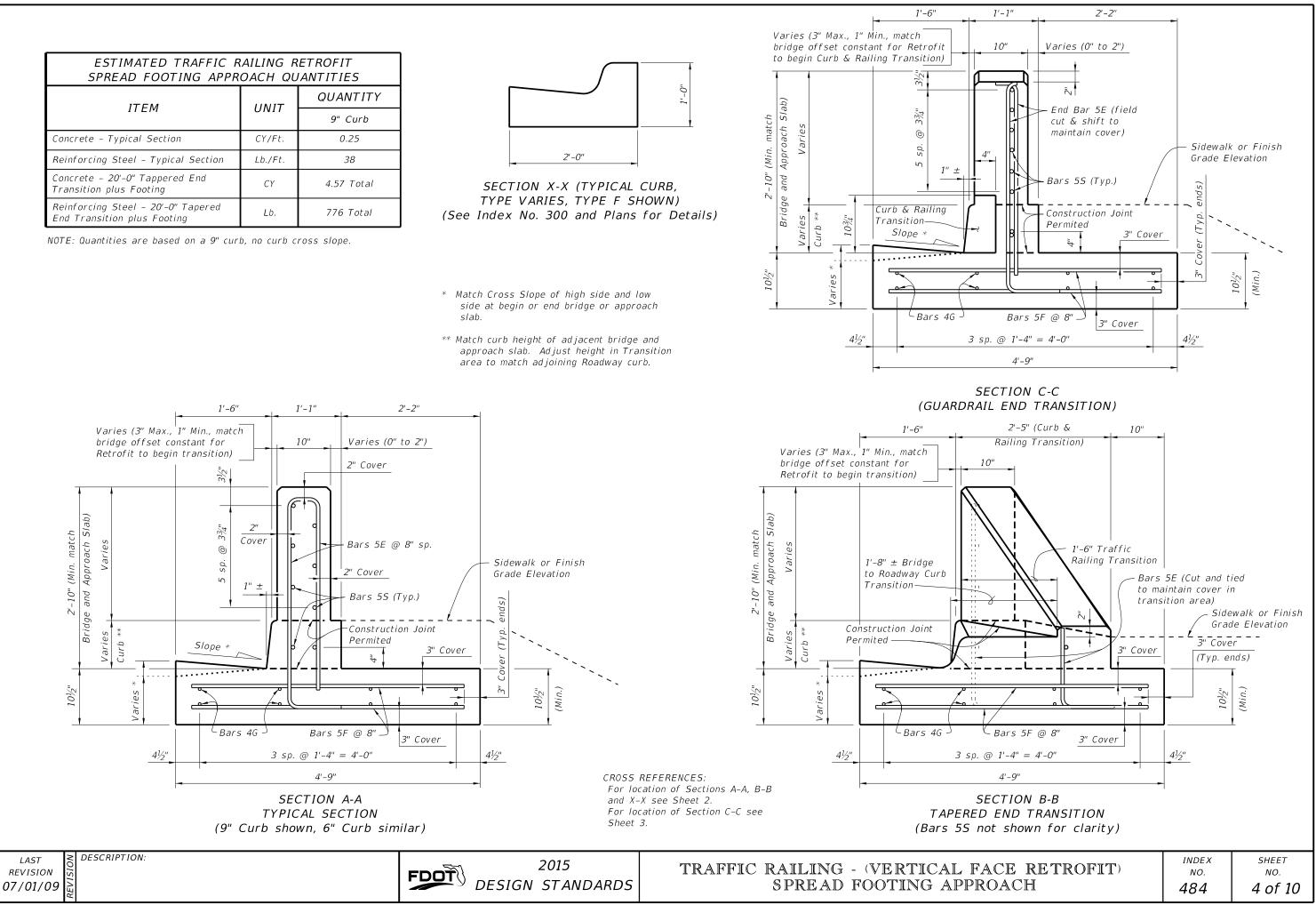
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES			
ITFM	UNIT		QUANTITY
11 EM		9" Curb	
Concrete – Typical Section	CY/Ft.	0.25	
Reinforcing Steel - Typical Section	Lb./Ft.	38	
Concrete - 20'-0" Tappered End Transition plus Footing	СҮ	4.57 Total	
Reinforcing Steel - 20'-0" Tapered End Transition plus Footing	Lb.	776 Total	

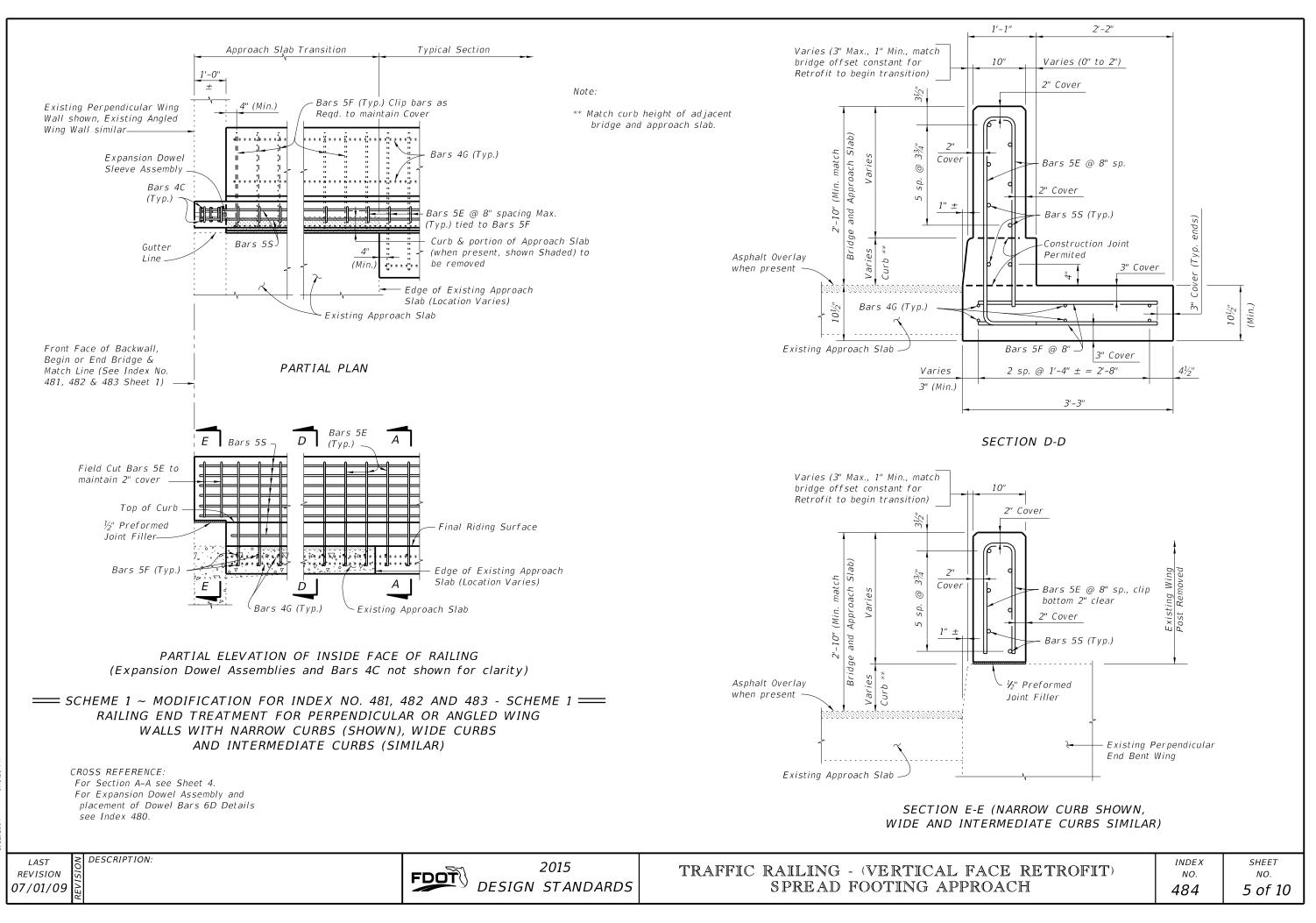


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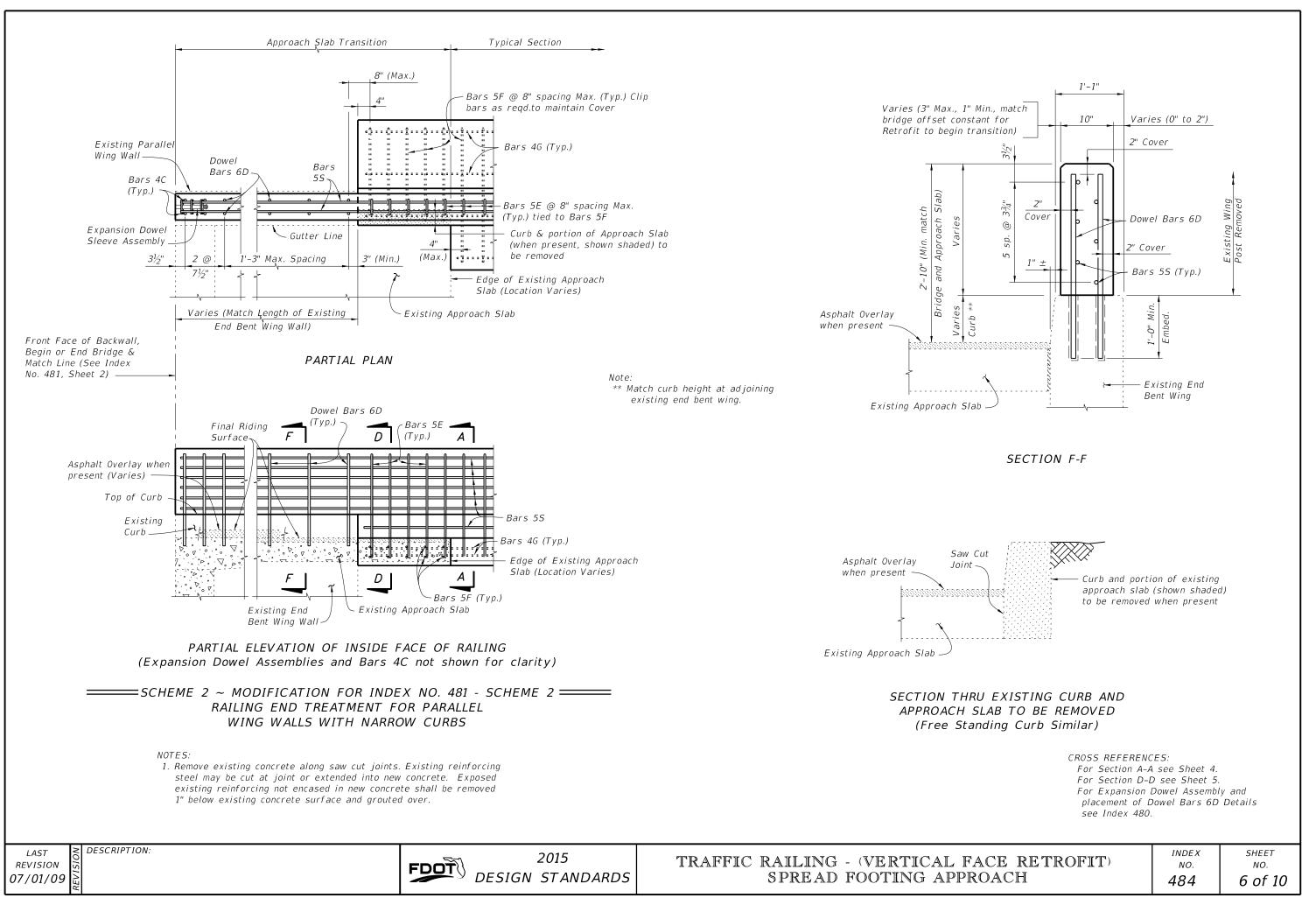


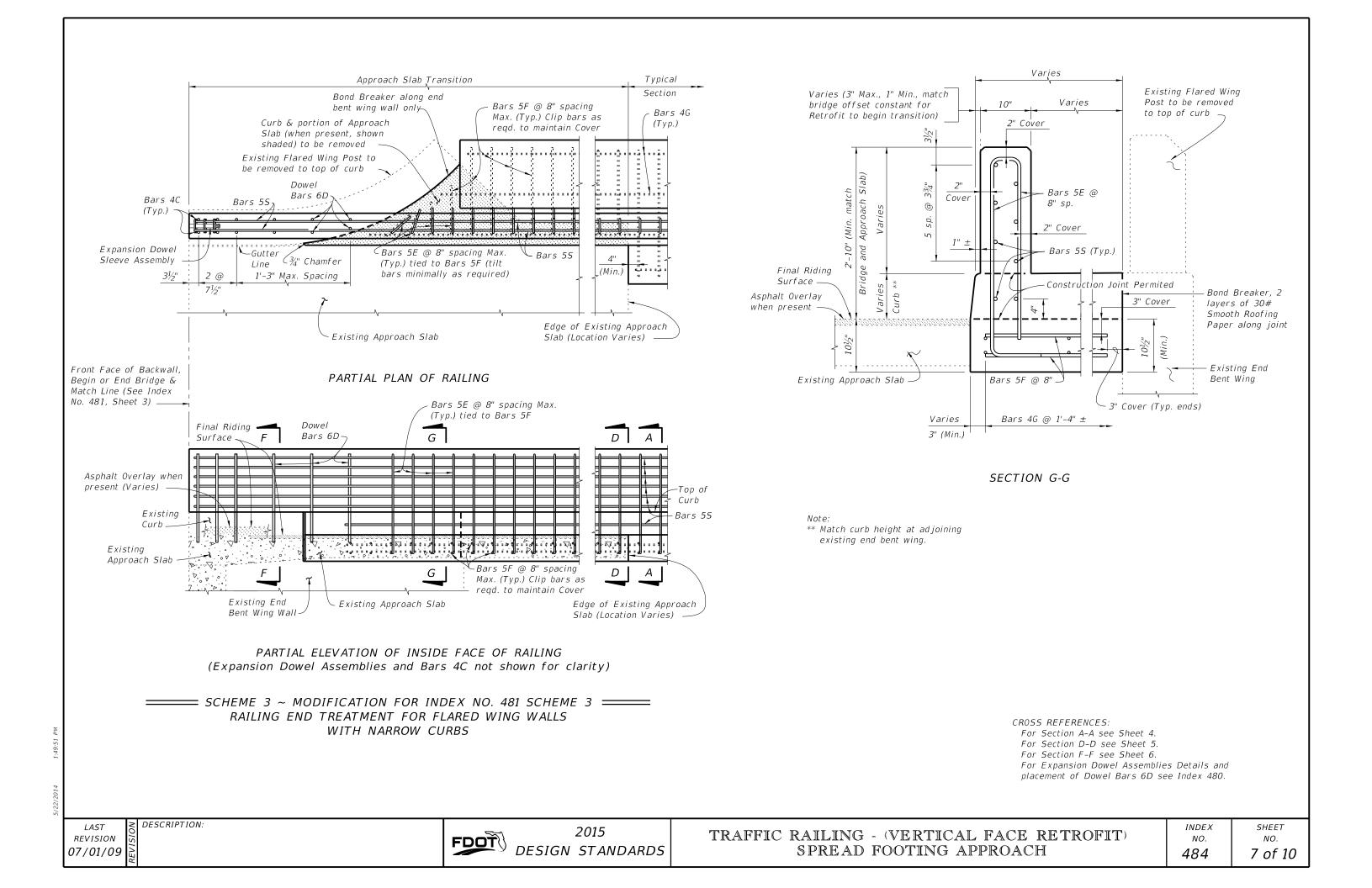
approach slab. Adjust height in Transition area to match adjoining Roadway curb.

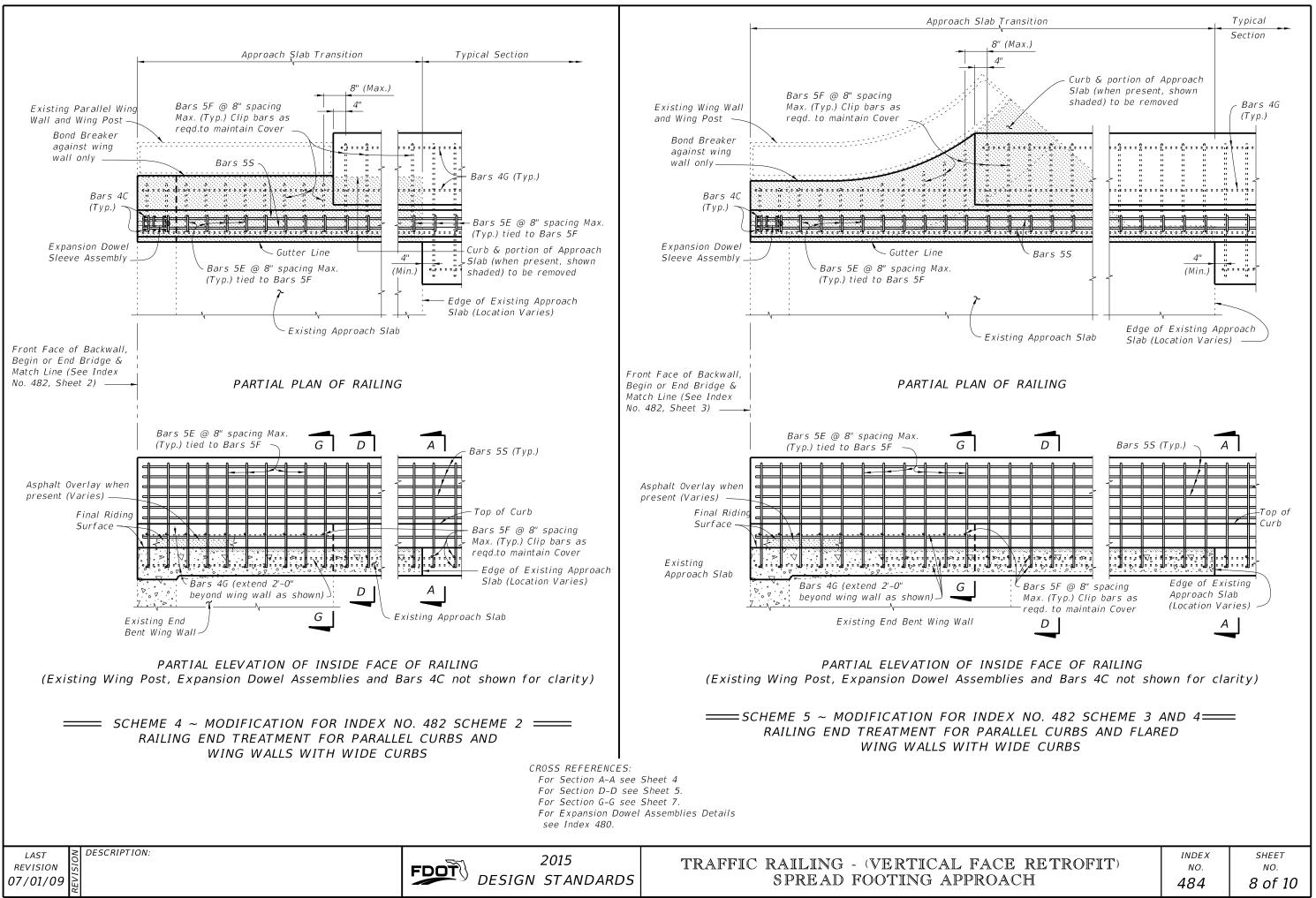




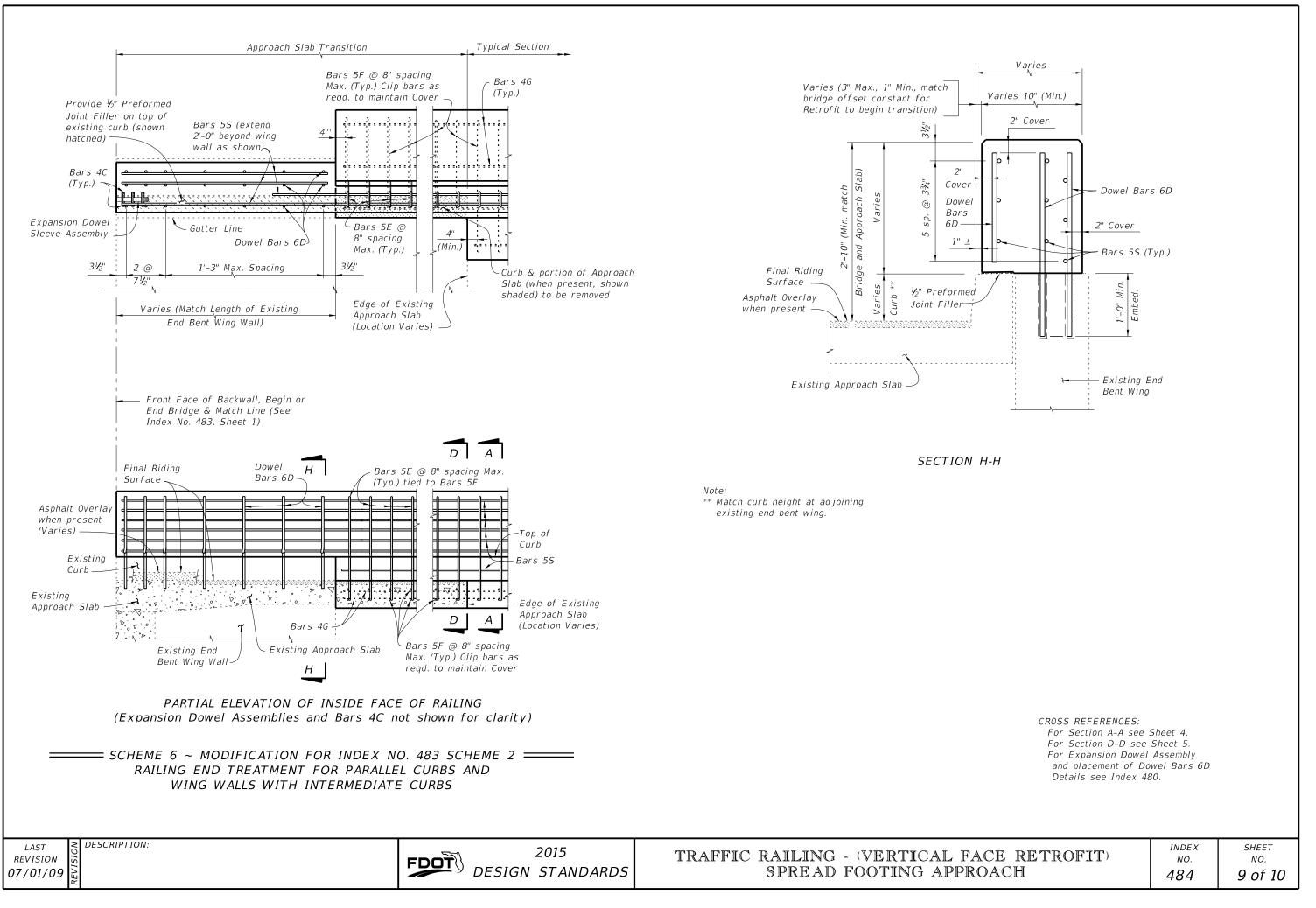
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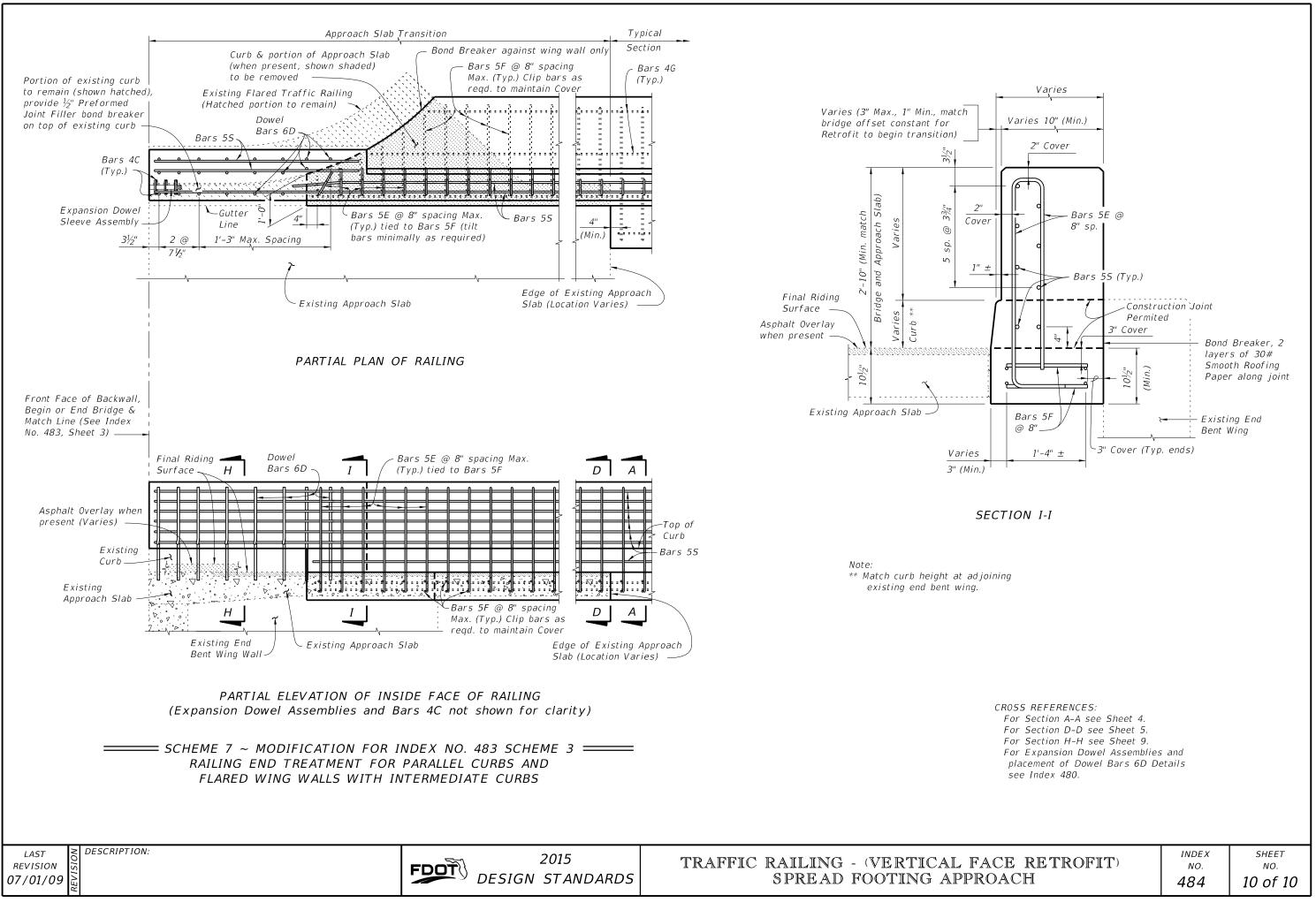




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