

3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 54" Height Similar)

TABLE 1 - RAILING MEMBERS						
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS			
Post "A"	$HSS2\frac{1}{2}x1\frac{1}{2}x\frac{1}{8}$	2.50" x 1.50"	0.125"			
Post "B"	HSS2½x1½x ³ / ₁₆	2.50" x 1.50"	0.188"			
Top Rail	2½" NPS (Sch. 10)	2.875"	0.120"			
	HSS3.000x0.120	3.000"	0.120"			
5 / //	2½" NPS (Sch. 10)	2.875"	0.120"			
End Hoops	HSS3.000x0.120	3.000"	0.120"			
Top Rail Joint/Splice Sleeves	HSS2.500x0.125	2.500"	0.125"			
Intermediate & Bottom Rail	HSS2x2x ³ ∕ ₁₆	2.00" x 2.00"	0.188" ⁽¹⁾			
Int. & Bottom Rail Post Connection Sleeve	HSS1.500x0.125	1.500"	0.125" ⁽¹⁾			
Handrail Joint/Splice Sleeves	1" NPS (Sch. 40) 1.315"		0.133"			
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"			
Handrail Support Bar	³¼" Ø Round Bar	0.750"	N/A			
Pickets (Type 1 Infill Panel)	¾" Ø Round Bar	0.750"	N/A			
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies			

TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be $1\frac{1}{4}$ " NPS (Sch. 40).

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LAST REVISION 07/01/14

DESCRIPTION:

FOOT DESIGN STANDARDS

= NOTES =

DESIGN LOADS, GEOMETRY AND APPLICABILITY:

See the Instructions for Design Standards for the design loads, geometry and applicability requirements. GENERAL:

Adequate foundation support shall be provided for anchorage and stability against overturning (See Sheet 8). See Index No. 851 for special requirements and modifications for use on bridges. The railing shown on these drawings requires a handrail for ramps steeper than a 5% grade to conform with the requirements of the Americans with Disabilities Act (ADA).

RAILS, PANELS AND POSTS:

Pipe Rails and Pickets shall be in accordance with ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) or ASTM A36 for bars. Structural Tube shall be in accordance with ASTM A500 Grade A, B, C or D, or ASTM A501. Perforated panels (Type 5), U-Channels and filler plates shall be ASTM A36 or A1011 (Grade 36). Posts and End Rails shall be fabricated and installed plumb, ± 1" tolerance when measured at 3'-6" above the foundation. Pickets and vertical panel elements shall be fabricated parallel to the posts, except that Type 2, 3 & 5 panel infills may be fabricated parallel to the longitudinal grade. Corners and changes in tangential longitudinal alignment shall be made continuous with a 9" bend radius or terminate at adjoining sections with mitered end sections when handrails are not required. For changes in tangential longitudinal alignment greater than 45°, posts shall be positioned at a maximum distance of 2'-0" each side of the corner and shall not be located at the corner apex. For curved longitudinal alignments the top and bottom rails and handrails shall be shop bent to match the alignment radius.

BASE PLATES AND RAIL CAPS:

Base Plates and Rail Cap Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36. SHIM PLATES:

Shim Plates shall be aluminum in accordance with ASTM B209, Alloy 6061 or 6063. Shim plates shall be used for foundation height adjustments greater than $\frac{1}{4}$ " and localized irregularities greater than $\frac{1}{8}$ ". Field trim shim plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of trimmed flat shim plates shown. Stacked shim plates must be bonded together with adhesive bonding material and limited to a maximum total thickness of $\frac{1}{2}$ ", unless longer anchor bolts are provided for the exposed thread length.

ANCHOR BOLTS:

Anchor bolts shall be in accordance with ASTM F1554 Grade 36. Headless anchor bolts for Adhesive Anchors shall be threaded full length. Cutting of reinforcing steel is permitted for drilled hole installation. Expansion Anchors are not permitted. All anchor bolts shall have single self-locking hex nuts. Tack welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only), shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, distort the anchor bolt threads to prevent removal of the nuts. Distorted threads and tack welds shall be coated with a galvanizing compound in accordance with the Specifications.

RESILIENT AND NEOPRENE PADS:

Resilient and Neoprene pads shall be in accordance with Specification Section 932 except that testing of the finished pads shall not be required. Neoprene pads shall be durometer hardness 60 to 80. JOINTS:

Grind welded joints as necessary to remove burs and weld splatter, additionally remove any sharp edges on rails to prevent injury. Grind all plug welds smooth. Expansion joints shall be spaced at a maximum 40'-0". Field splices similar to the expansion joint detail may be approved by the Engineer to facilitate handling, but top rail must be continuous across a minimum of two posts. For intermediate and bottom horizontal rails the screwed joints shown on Sheet 4 may be substituted with alternate joints shown on Sheet 3 Detail "K".

All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal shall be E60XX or E70XX. Nondestructive testing of welds is not required.

COATINGS:

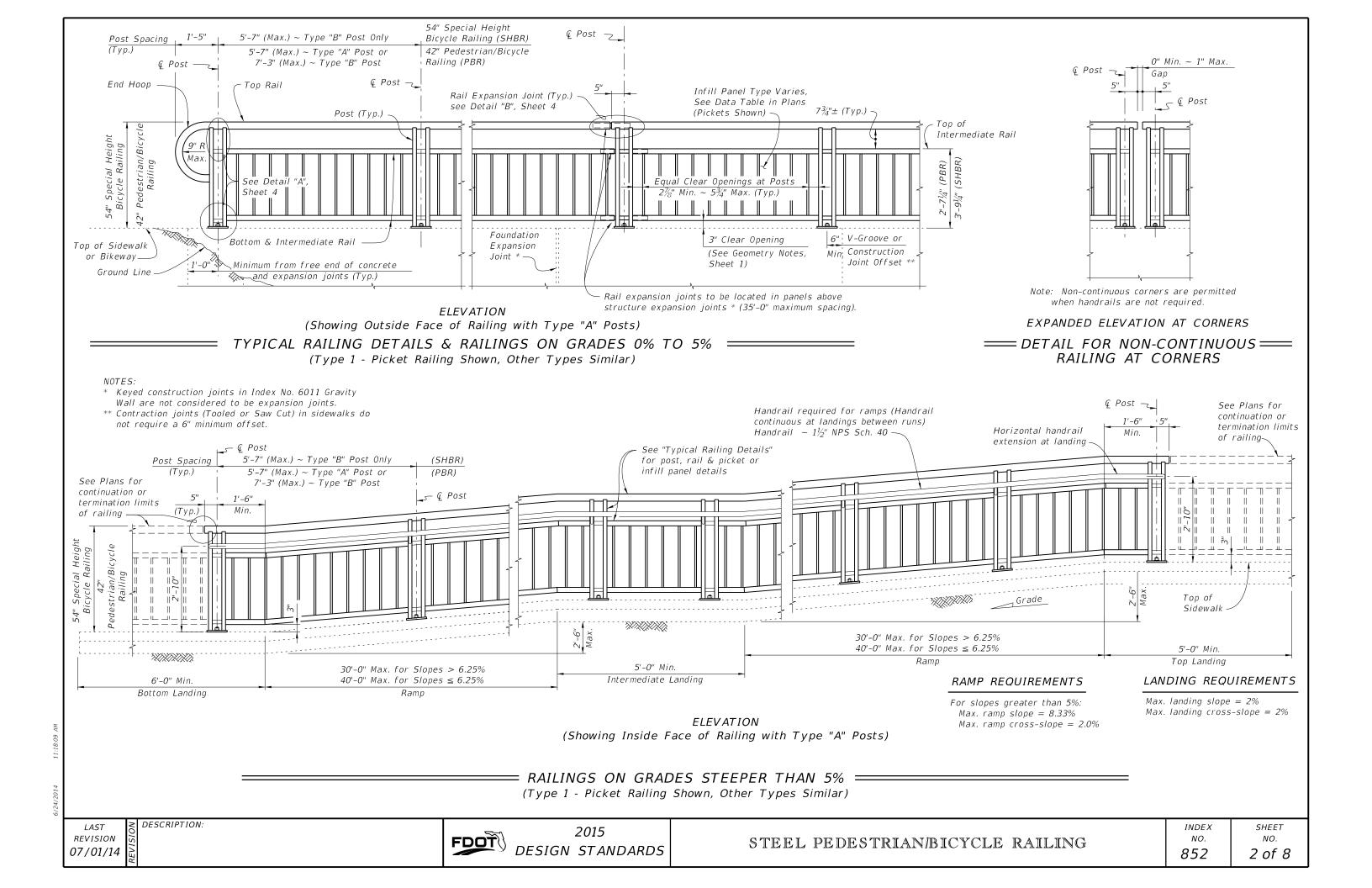
The steel railing shall be hot-dip galvanized after fabrication in accordance with Section 962 of the Specifications. All nuts, bolts and washers shall be hot-dip galvanized in accordance with Specification Section 962.

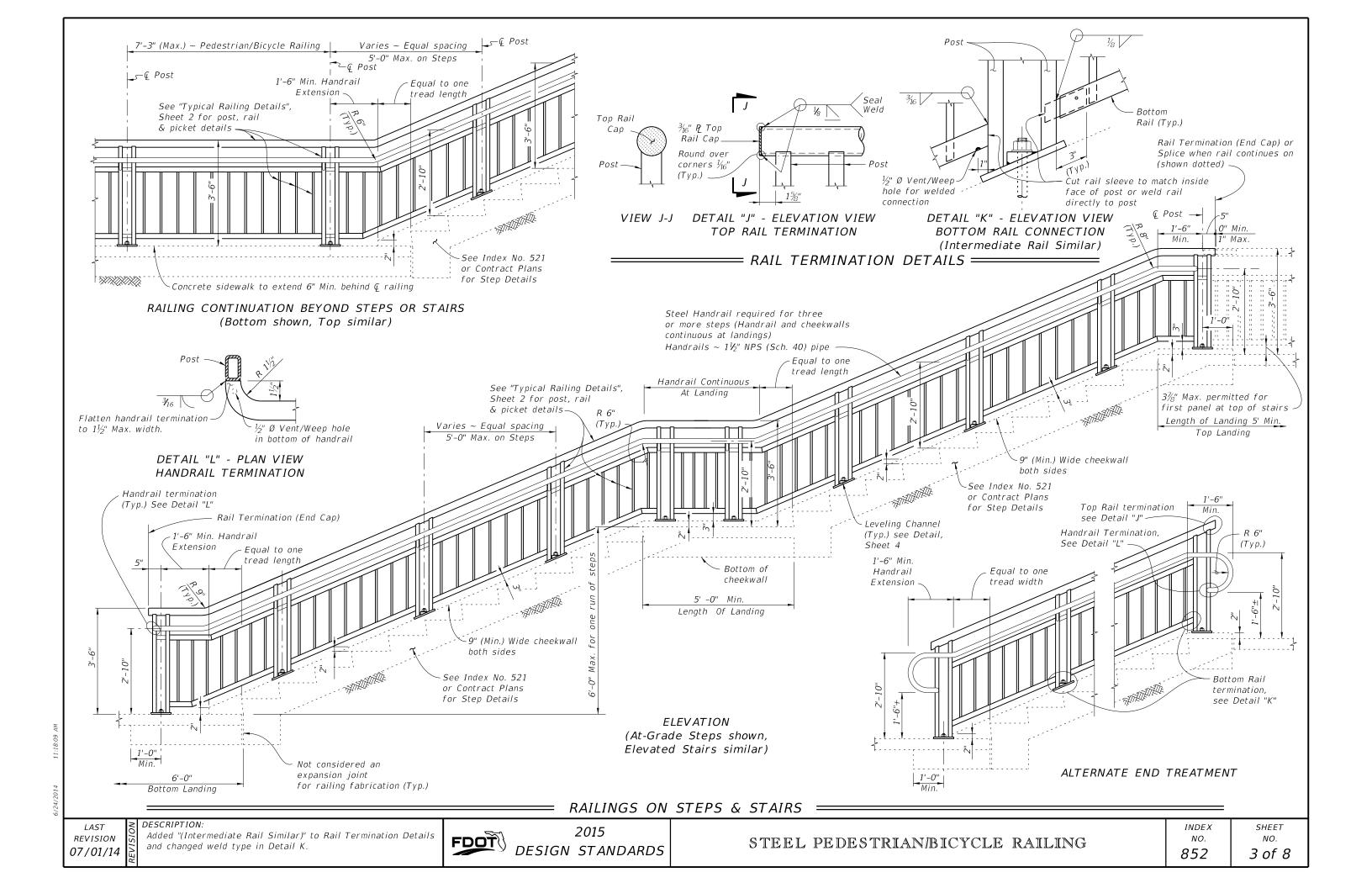
SHOP DRAWINGS:

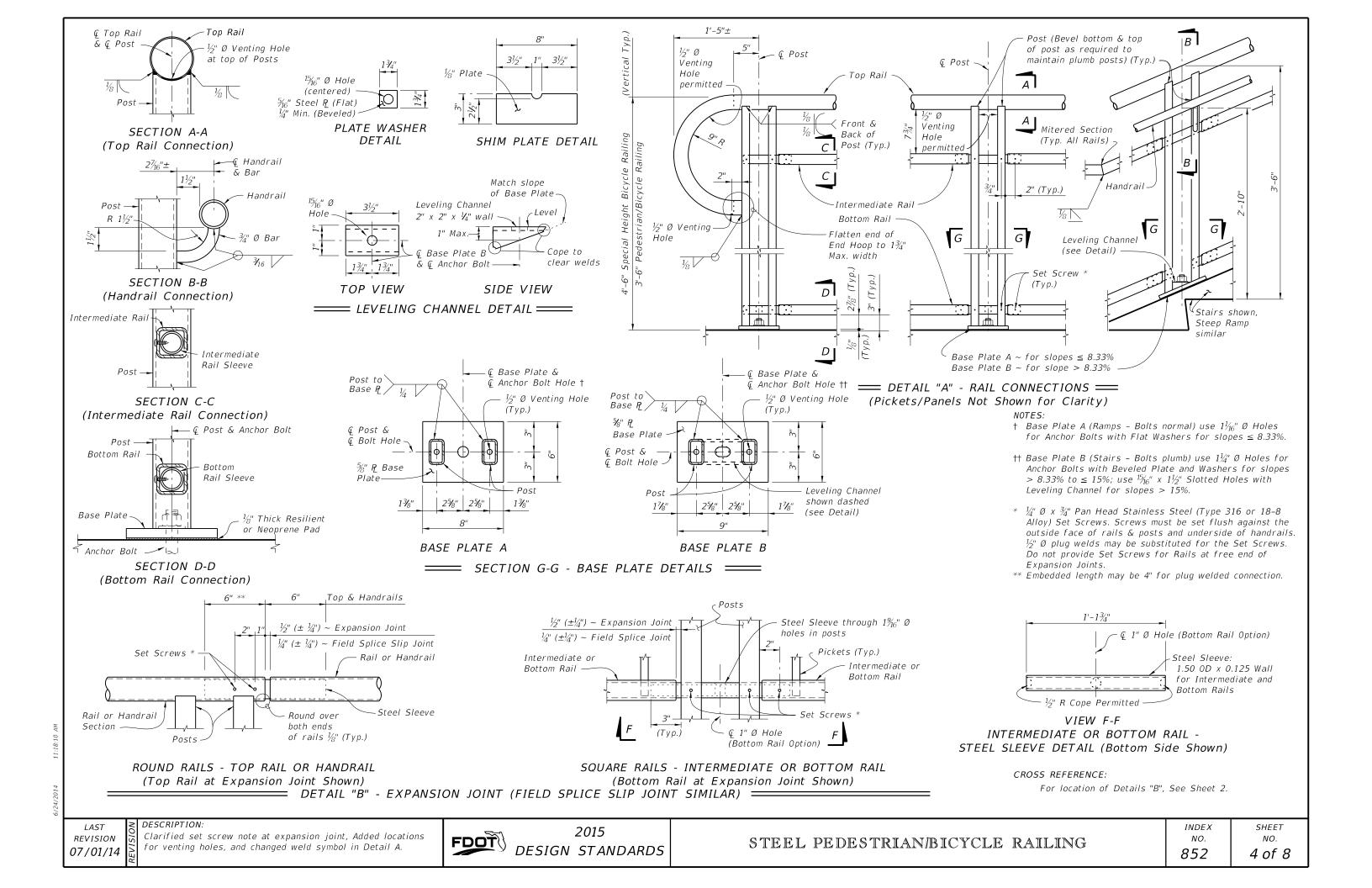
Details addressing project specific geometry (line & grade) showing post and expansion joint locations, post and panel type, anchor bolt installation "Case" or lengths, must be submitted by the Contractor for the Engineer's approval prior to fabrication of the railing. Shop drawings shall be in accordance with the Specifications.

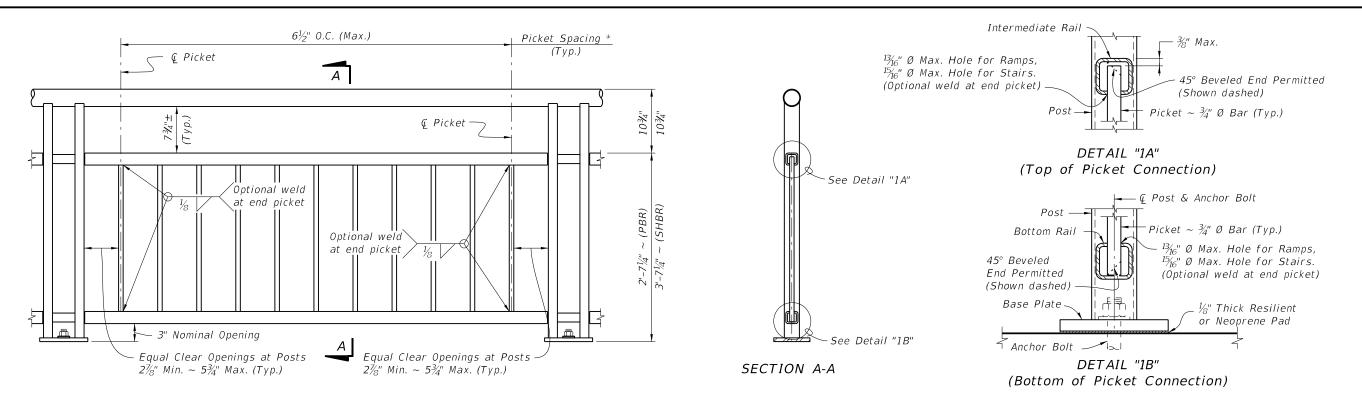
PAYMENT.

Railing shall be paid for per linear foot (Item No. 515-2-abb). Payment will be plan quantity measured as the length along the center line of the top rail, and includes rails, posts, pickets, panels, rail splice assembly, base plates, anchor bolts, nuts, washers, resilient or neoprene pads and all incidental materials and labor required to complete installation of the railing.





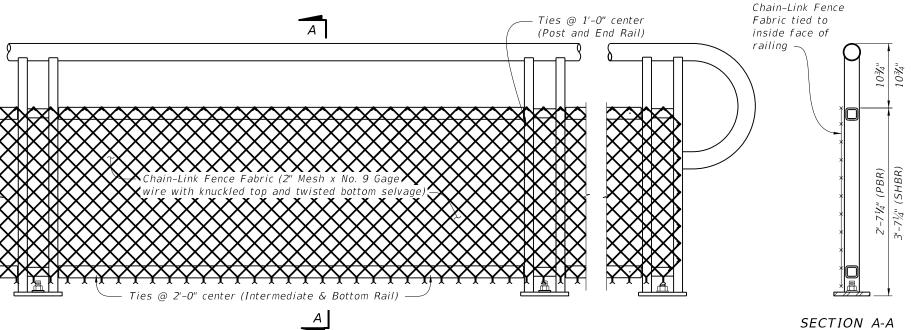




TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. If an alternate design is used, maintain a maximum clear opening of $5\frac{7}{8}$ " for standard installations and 3%" for special conditions.



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TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS				
COMPONENT	ASTM	COMPONENT INFORMATION		
Chain-Link Fence Fabric (2" mesh with twisted bottom and knuckled top selvage)	A 392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating		
	A 491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)		
	F 668	Polyvinyl Chloride (PVC) Coated Steel - 9 gage Zinc-Coated Wire (metallic-coate core wire diameter) ~ See Plans for specified color of PVC.		
Tie Wires	F 626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.		
Tension Bars	F 626	$rac{y_{16}''}{16}$ (Min. thickness) x $rac{y_4''}{4}$ (Min. width) x 2'-3' (Min. height) Steel Bars		
Miscellaneous Fence Components	F 626	Zinc-Coated Steel		

N-LINK PANEL NOTE:

in-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

1. See Plans for Infill Panel option required.

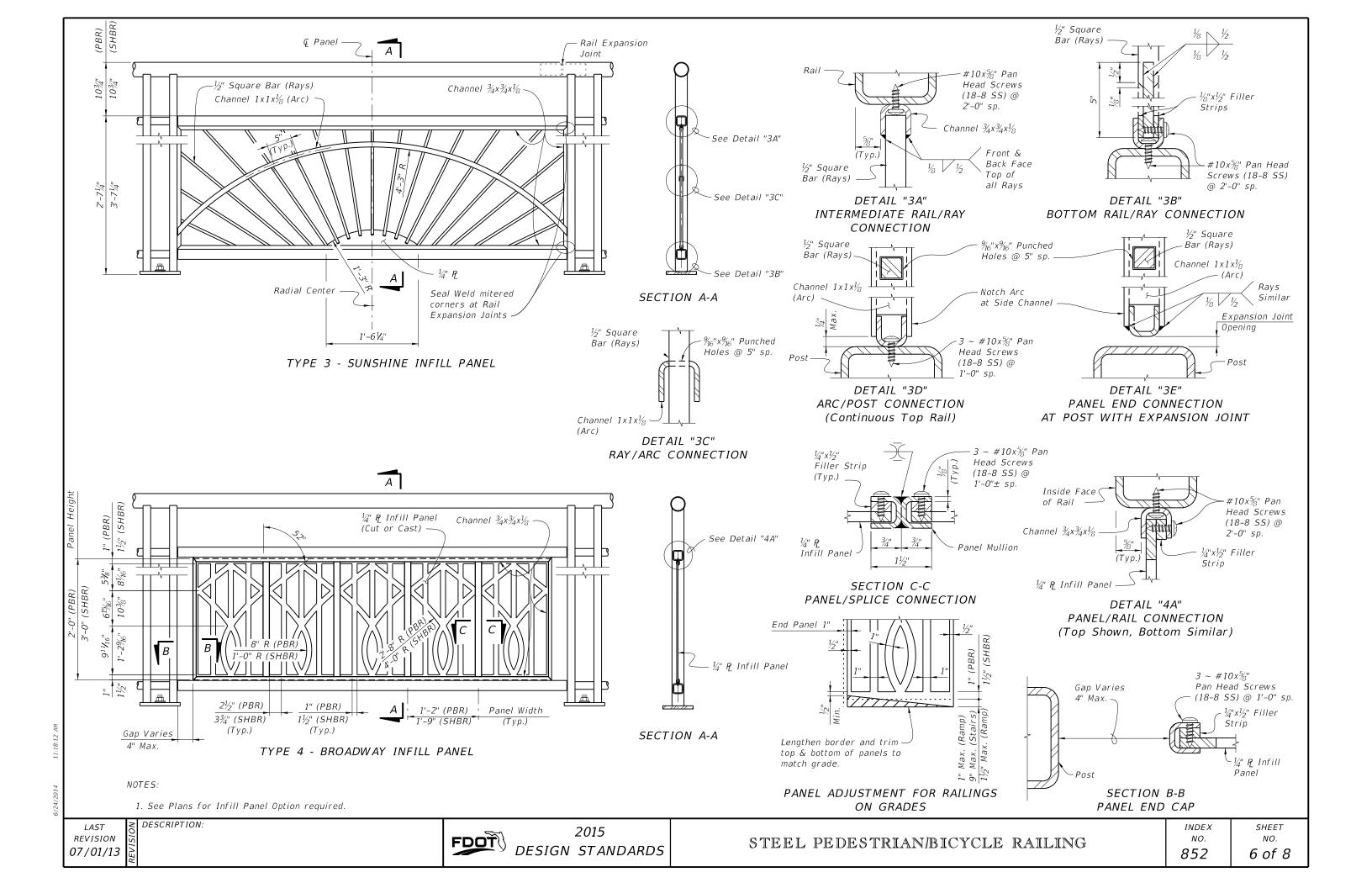
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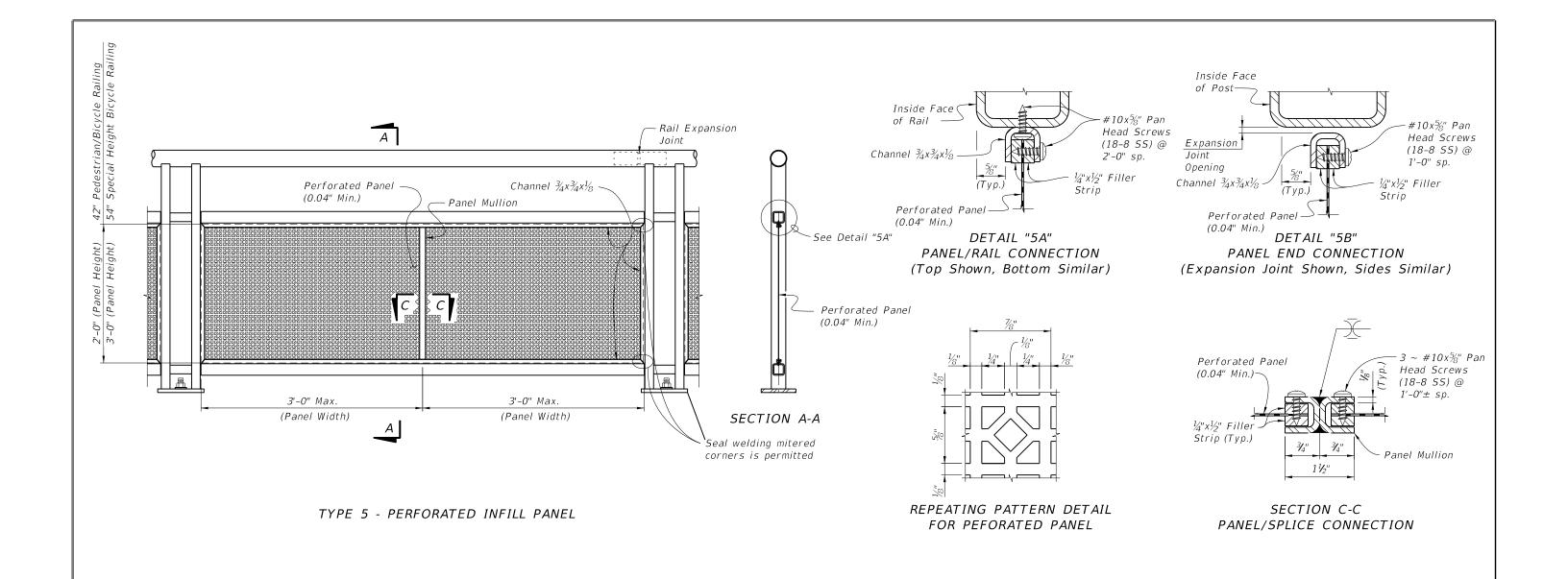
DESCRIPTION: Corrected dimension for SHBR Type 1.



STEEL PEDESTRIAN/BICYCLE RAILING

INDEX SHEET NO. NO. 852 5 of 8



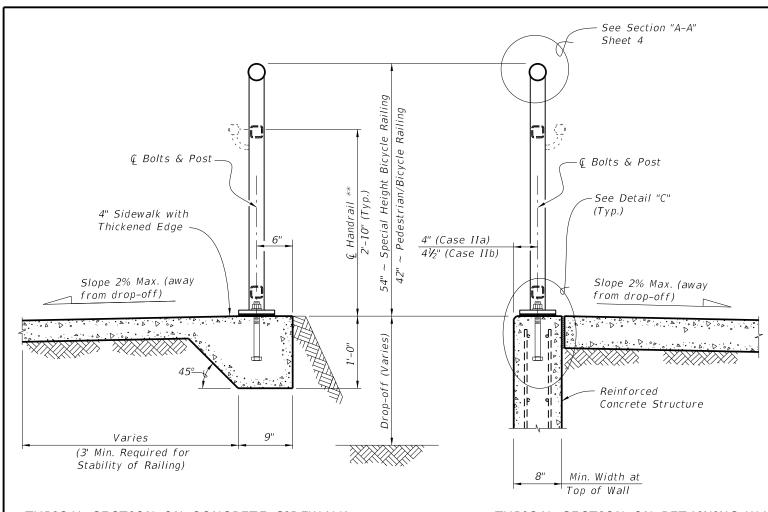


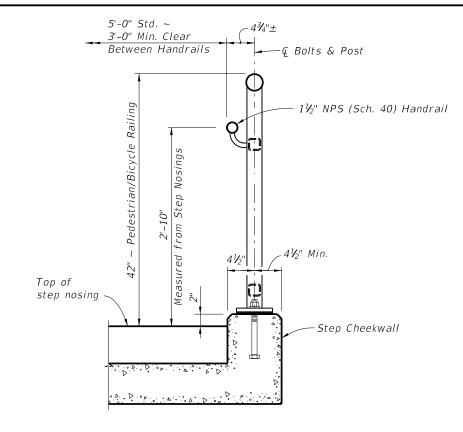
NOTES:

1. See Plans for Infill Panel Type required.

≥ DESCRIPTION: LAST REVISION 07/01/13

2015 FDOT DESIGN STANDARDS





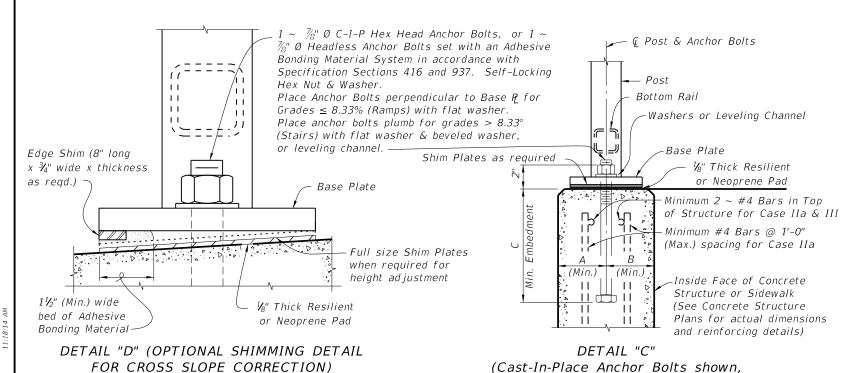
TYPICAL SECTION ON STEPS & STAIRS (Case III)

TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

(Used in lieu of Beveled Shim Plates)

≥ DESCRIPTION:

TYPICAL SECTION ON RETAINING WALL (Case II)



	ANCHOR BOLT TABLE							
6465	CTRUCTURE	DIMENSIONS		ANCHOR LENGTH		ANGUOD		
CASE	STRUCTURE TYPE		B Edge Dist.	C Embedment	C-I-P Hex Head Bolt	Adhesive Anchor	ANCHOR SIZE	
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	%" Ø	
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	%" Ø	
IIb	Gravity Wall Index No. 6011	4½"	3½" @ top	1'-0" *	1'-1½"	1'-2"	%" Ø	
III	Step Cheekwall	4½"	4½"	9"	10½"	11"	%" Ø	

- * Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".
- ** When required; measured from top of sidewalk.

LAST REVISION 07/01/14



Adhesive Anchors similar)