

(Shear key shown dashed)

NOTES

DESIGN SPECIFICATIONS:

Design according to FDOT Structures Manual (current edition).

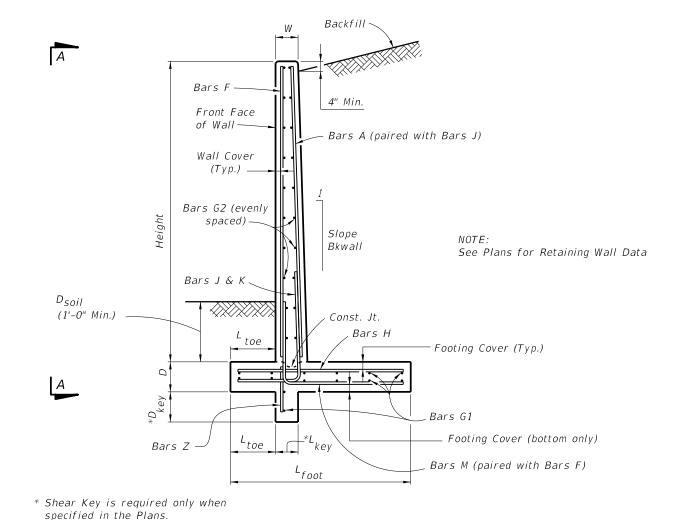
TRAFFIC RAILINGS OR PARAPETS:

If there is a Traffic Railing or Parapet on the wall, align Wall Joints with V-Grooves, and Wall Expansion Joints with Barrier Open Joints.

FOUNDATION: Prepare the soil below the footing in accordance with the requirements for spread footings in Specification Section 455.

PAYMENT:

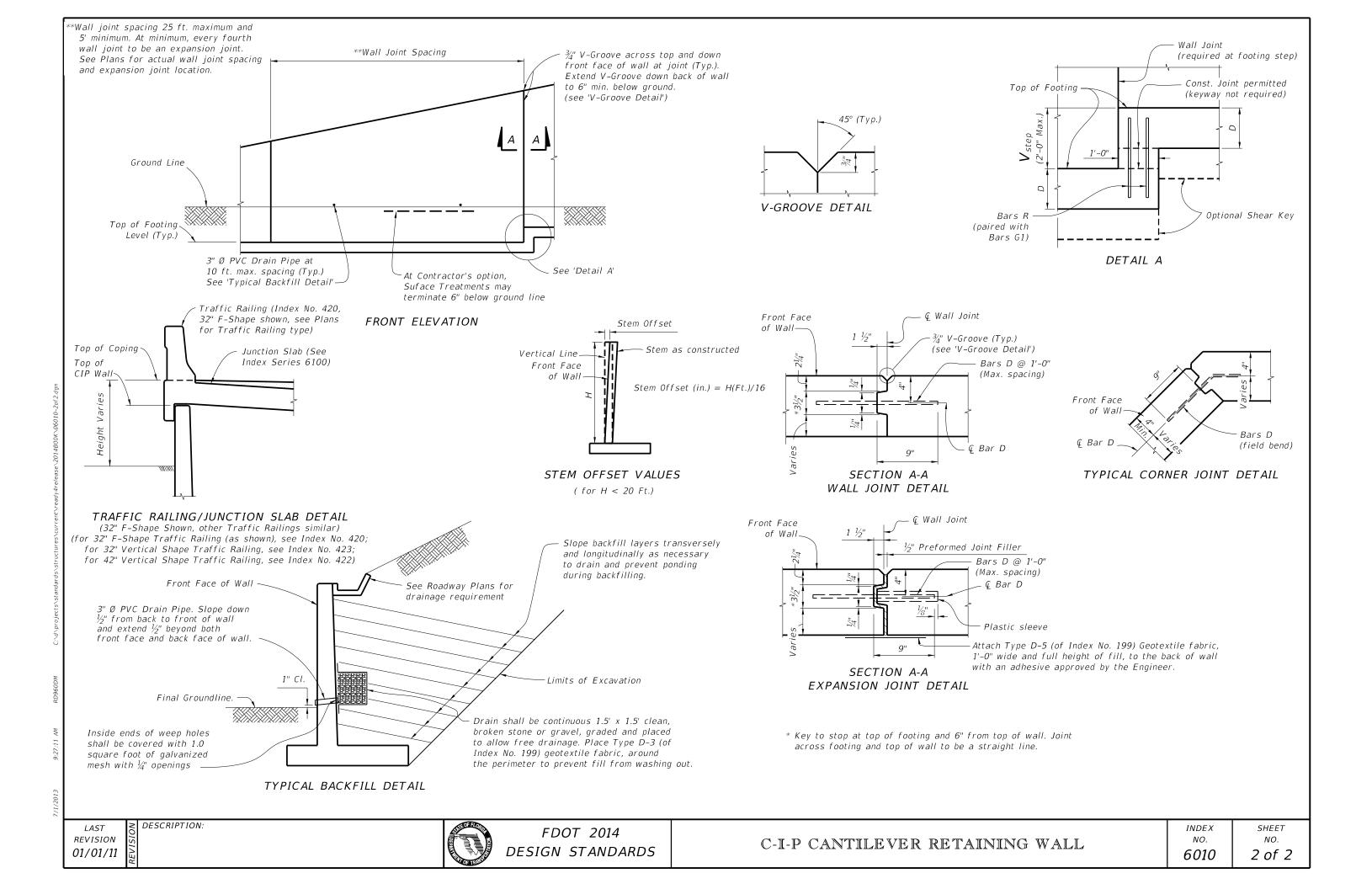
All Retaining Wall costs, including all miscellaneous costs, shall be paid for at the unit contract price for either Class II, III or IV Concrete (Retaining Walls) (CY) and Reinforcing Steel (Retaining Walls) (lbs.). Retaining Wall quantities shall not include concrete nor reinforcing steel for Traffic Railing, Parapet or Junction Slab.

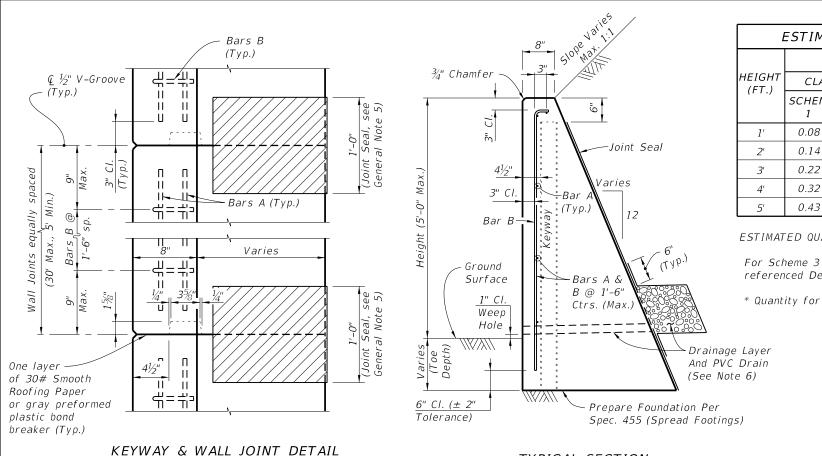


TYPICAL SECTION

REINFORCING STEEL BENDING DIAGRAMS Total Length <u>1'-0"</u> Lap Splice (Typ.) BARS G1 Slope Bkwall В BARS J & K BARS M NOTE: All bar dimensions are out-to-out







(TOP VIEW)

ESTIMATED QUANTITIES FOR WALL PER LINEAR FOOT OF WALL WFFP HOLES CLASS NS CONCRETE (CY) REINF. & DRAIN STEEL SCHEME SCHEME SCHEME REOD. (LB.) 2 3 0.08 0.11 (0.20*) 0.03 3 (4*) No 0.14 0.20 (0.32*) 0.09 4 (5*) Nο 0.22 0.32 (0.47*) 0.29 5 (6*) Yes 0.32 0.47 (0.65*) 0.43 6 (7*) Yes

ESTIMATED QUANTITIES NOTES:

For Scheme 3 Junction Slab and Traffic Railing see the referenced Design Standards for estimated quantities.

0.60

7 (8*)

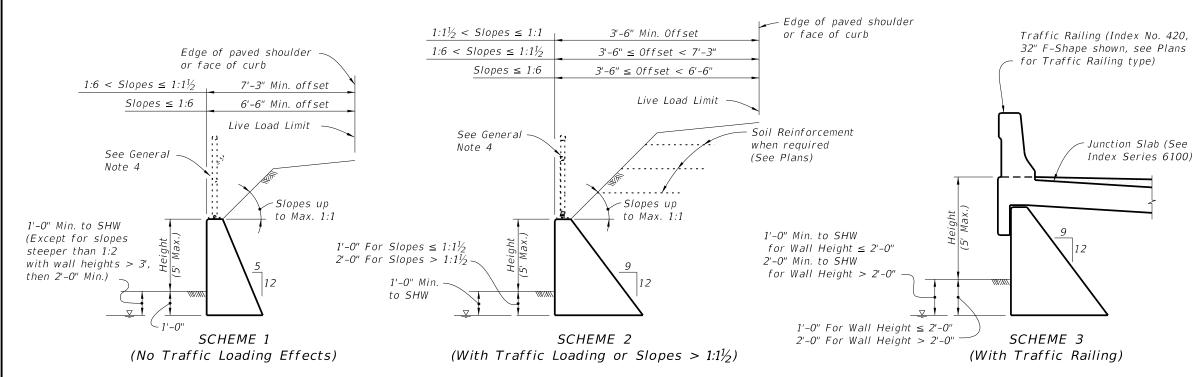
Yes

* Quantity for 2'-0" Toe Depth in Scheme 2.

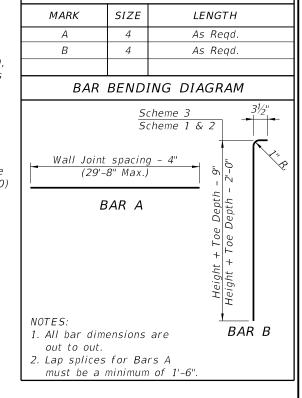
0.65 (0.85*)

GENERAL NOTES

- 1. Gravity walls constructed as extensions of reinforced concrete retaining walls, except walls of proprietary designs, shall have the same face texture and finish as the reinforced concrete retaining wall.
- 2. Concrete for Gravity Wall shall be Class NS per Section 347. Concrete for Scheme 3 Junction Slab and Traffic Railing shall be Class II per Section 346, unless otherwise specified in the plans.
- 3. Reinforcing steel shall meet the requirements of Specification Section 931 (Grade 40 or 60) and be provided at the max. spacings shown. Smooth or Deformed Welded Wire Reinforcemen (WWR) may be substituted on an equal area basis. Do not increas bar/wire spacing for Grade 60 reinforcing steel or WWR.
- 4. When required, for adjunct guiderail see the plans, Index No. 870 or 880 as appropriate. For adjunct Type B fence see Index No. 802.
- 5. Joint seal to be two layers of 30# smooth roofing paper or Type D-5 geotextile fabric in accordance with Index No. 199. Mop all contact surfaces of concrete and roofing paper or geotextile fabric with cut-back asphalt. Stop roofing paper or geotextile fabric 6" below top of wall).
- 6. Provide a continuous 1'x1' clean gravel or crushed rock drain for wall heights 3 ft. and higher. Wrap drainage layer as shown, with Type D-3 geotextile fabric in accordance with Index No. 199. Provide 8"x8" galvanized mesh with $\frac{1}{4}$ " openings, at the inside end of the PVC Drain Pipe. Provide 2" Ø PVC Drain Pipe (Sch. 40) at 10 ft. max. spacing (When Drainage Layer required). Locate minimum 2'-0" clear of wall joints.
- 7. Cost of reinforcing steel, face texture, finish, joint seal, drain pipes, drainage layer, galvanized mesh and geotextile fabric to be included in the Contract Unit Price for Concrete Class NS, Gravity Wall. Cost of concrete for Junction Slab in Scheme 3, to be included in Contract Unit Price for Concrete Traffic Railing Barrier With Junction Slab. Adjunct railings or fences to be paid for separately.



TYPICAL SECTION



BILL OF REINFORCING STEEL

LAST REVISION 07/01/13

DESCRIPTION:

FDOT 2014 **DESIGN STANDARDS**

C-I-P GRAVITY WALL

INDEX SHEET NO. NO. 6011 1 of 1

- The Florida Department of Transportation "Standard Specifications for Road and Bridge Construction", Current Edition and Supplements as Amended.
- 2. Design Specifications:
 - a. Florida Department of Transportation (FDOT) "Structures Design Guidelines", Current Edition
 - b. American Association of State Highway and Transportation Officials (AASHTO) "LRFD Bridge Design Specifications", Current Edition.
 - c. AASHTO-AGC-ARTBA Task Force 27 (Ground Modification Techniques), "Insitu Soil Improvement Techniques", January 1990.

DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and Chapter 3 of the FDOT Structures Design Guidelines.
- 2. It is the responsibility of the Engineer of Record to determine that the maximum factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer of Record.
- 4. If there are manholes and/ or drop inlets present, design and analysis for both internal and external stability shall be considered.

SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

MATERIALS:

- 1. Concrete Class: See Wall Control Drawings.
- 2. See Specification Section 548 for material requirements.
- 3. For additional material requirements see the Wall Company's General Notes.

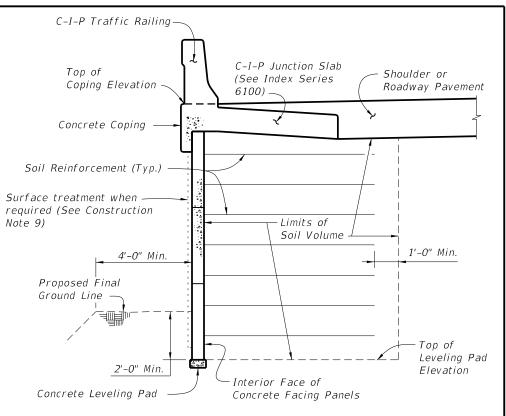
CONSTRUCTION:

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings. 3. If present, consider in design and analysis and locate manholes and drop
- inlets as shown on wall elevations. 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement
- strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements. 5. The Contractor is responsible for controlling water during storm events as
- needed during construction. 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the quardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be
- approved by the Engineer. 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 8. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.

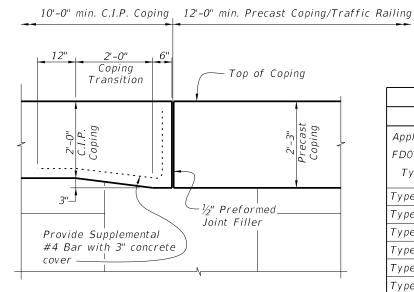
- 9. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 10. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 11. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 12. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- 13. The top of the leveling pad or footing will be 2'-0" minimum below final around line
- 14. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 15. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 16. Work this Index with Index 6100 & 6200 Series.

SHOP DRAWING REQUIREMENTS:

See Specification Section 548 for shop drawing requirements.



TYPICAL MSE RETAINING WALL SECTION WITH A TRAFFIC RAILING (Showing Limits of the Reinforced Soil Volume)



ELEVATION VIEW OF COPING HEIGHT TRANSITION (Railing Not Shown For Clarity)

	FDOT MSE RETAINING WALL CLASSIFICATION TABLE									
	Durability Requirements Other Allowable FDOT Wall Types								Types	
Applicable	Concrete	Concrete	Pozzolan	Soil						
FDOT Wall	Cover	Class	Additions?	Reinforcement	2A	2B	2C	2D	2E	2F
Type *	(in.)	for Panels	**	Type						
Type 2A	2	II	No	Metal		/	/	/	/	/
Type 2B	2	IV	No	Metal			/	1	/	1
Type 2C	3	IV	No	Metal				1	1	/
Type 2D	3	IV	Yes	Metal					/	/
Type 2E	3	IV	No	Plastic						/
Type 2F	3	IV	Yes	Plastic						

- * See Data Table in Contract Plans.
- ** Silica fume, metakaolin or ultrafine fly ash.

GENERAL NOTES AND DETAILS

LAST REVISION 07/01/13

FDOT 2014 **DESIGN STANDARDS**

PERMANENT MSE RETAINING WALL SYSTEMS

INDEX SHEET NO. NO. 6020 1 of 1

- The Florida Department of Transportation "Standard Specifications for Road and Bridge Construction", Current Edition and Supplements as Amended.
- 2. Design Specifications:
 - a. Florida Department of Transportation (FDOT) "Structures Design Guidelines", Current Edition.
 - b. American Association of State Highway and Transportation Officials (AASHTO) "LRFD Bridge Design Specifications", Current Edition.
 - c. AASHTO-AGC-ARTBA Task Force 27 (Ground Modification Techniques), "Insitu Soil Improvement Techniques", January 1990.

DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and FDOT Structures Design Guidelines Section 3.13.2.
- 2. It is the responsibility of the Engineer to determine that the factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer.
- 4. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.

SOIL PARAMETERS:

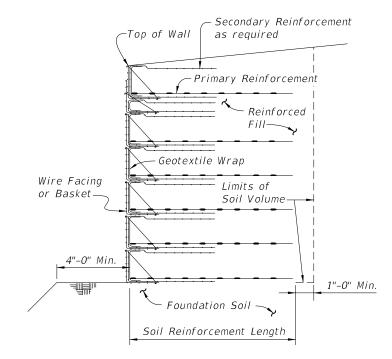
1. See wall control drawings for soil characteristics of foundation material to be used in the design of the wall system. The Contractor must provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site. Provide the values of unit weight, cohesion and internal friction angle in the Shop Drawings.

MATERIALS

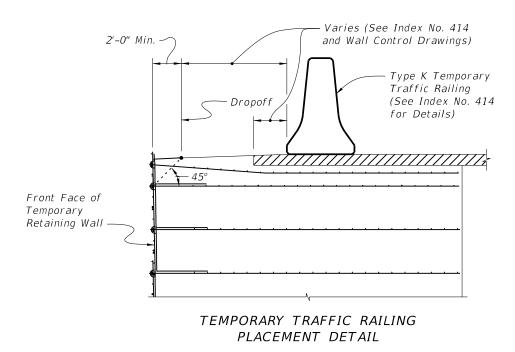
- 1. Provide soil reinforcement in accordance with Specification Section 548.
- 2. For additional material notes, see Wall Company General Notes.

CONSTRUCTION:

- 1. Walls must be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. Refer to Plan and Elevation sheets of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 4. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor must notify the Engineer to determine what course of action should be taken.
- 5. The Contractor is responsible for gradually deflecting upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.



TYPICAL RETAINING WALL SECTION (Showing Limits of the Reinforced Soil Volume)

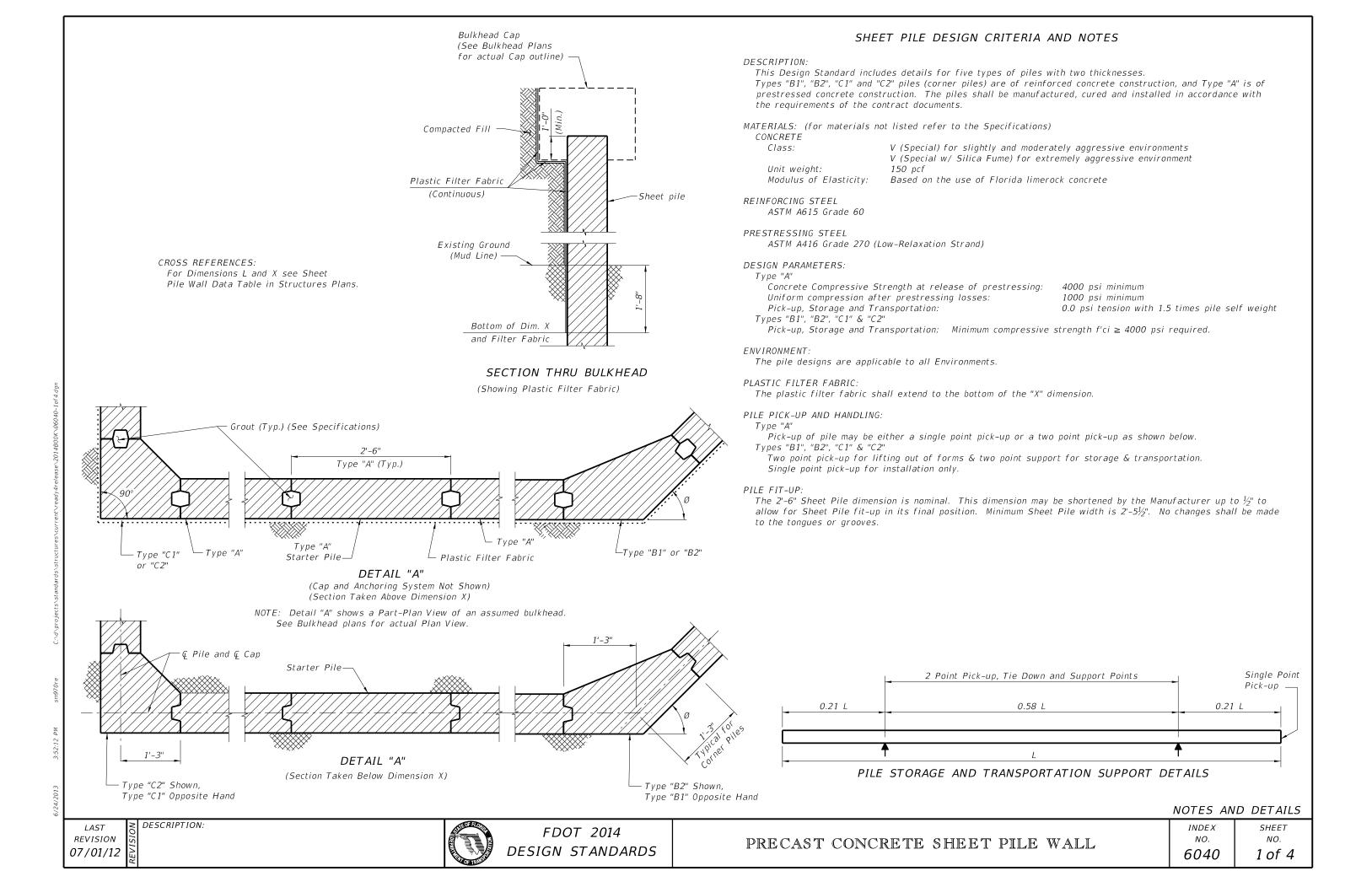


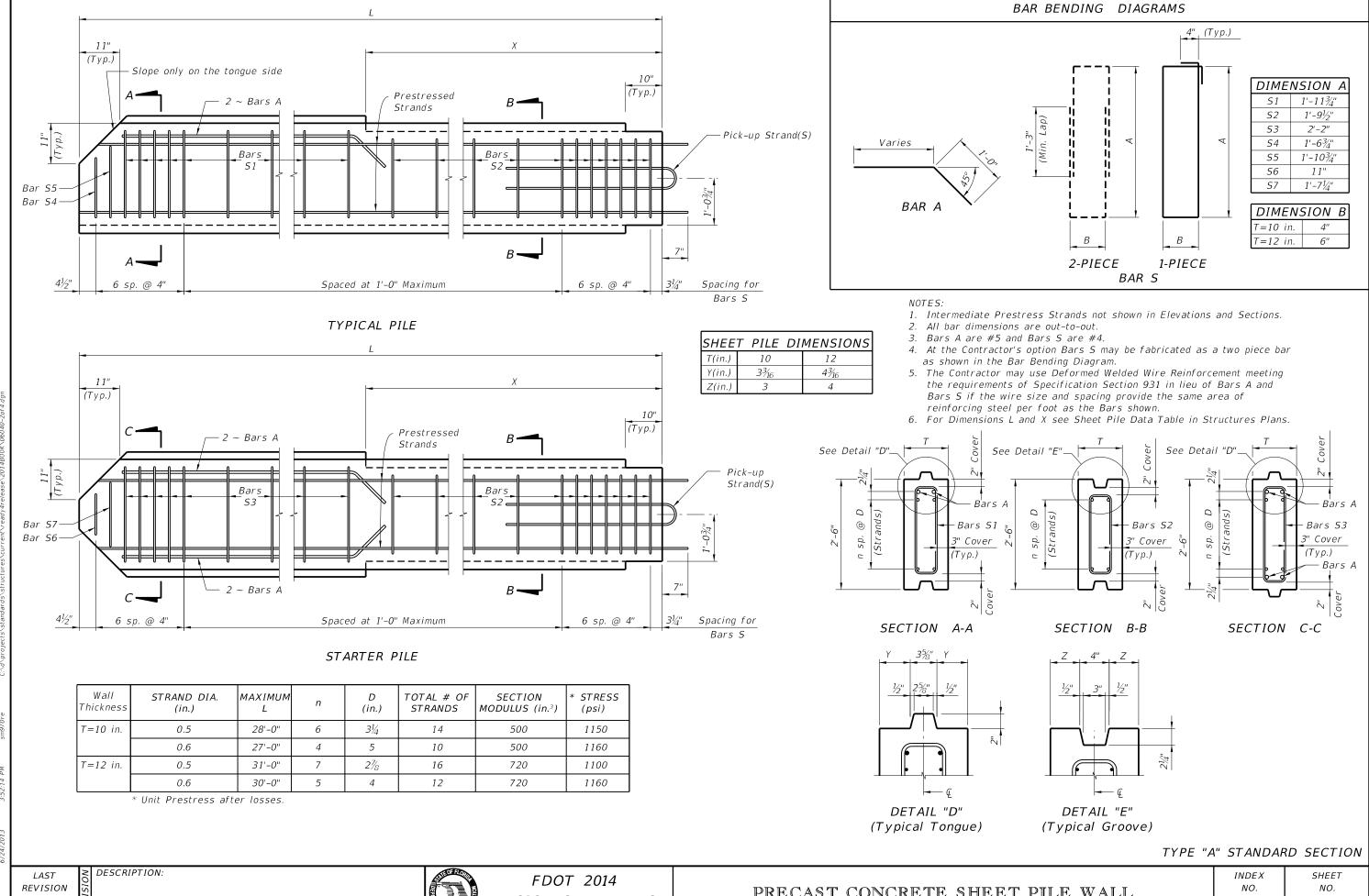
GENERAL NOTES AND DETAILS

6/24/2013

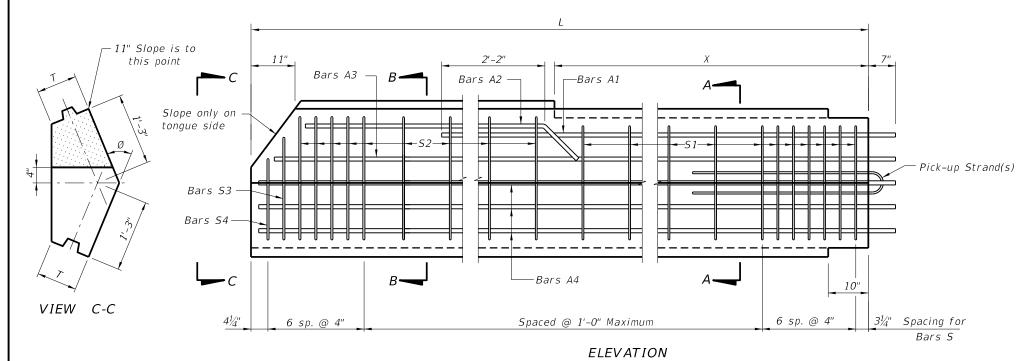
DESCRIPTION:







07/01/13

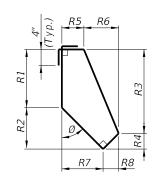


(TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

BAR BENDING DIAGRAMS

	STIRRUP DIMENSIONS (T = 10")								
Ø	BAR MARK	R1	R2	R:3	R4	R·5	R·6	R7	R8
	<i>S</i> 1	11½"	9¾"	1'-6½"	2½"	5"	4¾"	5½"	41/4"
30°	S-2	1'-1½"	9¾"	1'-8¾"	2½"	4½"	5½"	5¾"	41/4"
30	<i>S3</i>	1 1½"	8"	1'-6"	11/4"	5"	4½"	4½"	5"
	54	1 1½"	41/4"	1'-1 ³ / ₄ "	13/4"	5"	3¾"	2½"	$6\frac{1}{4}$ "
	<i>S</i> 1	11½"	8"	1'-4"	4"	5½"	6½"	8"	4"
45°	52	1'-1 ³ / ₄ ''	8"	1'-5¾"	4"	4½"	7½"	8"	4"
73	53	11½"	6¾"	1'-4"	2½"	5½"	6¾"	$6\frac{3}{4}$ "	5½"
	54	11½"	3½"	1'-0"	3"	5½"	5"	3½"	7"
	<i>S</i> 1	1'-0"	6"	1'-0¾"	5½"	6"	7½"	101/4"	3"
600	S-2	1'-2"	6"	1'-2¾"	5½"	43/4"	8¾"	10½"	3"
60°	53	1'-0"	4¾"	1'-1½"	3½"	6"	8"	8¾"	5½"
	54	1'-0"	2½"	10"	4½"	6"	5¾"	4"	7½"

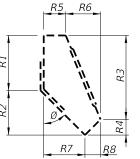
STIRRUP DIMENSIONS $(T = 12")$									
Ø	BAR MARK	R1	R2	R·3	R4	R5	R·6	R7	R8
	<i>S</i> 1	11½"	10"	1'-6"	3½"	7"	43/4"	5 ³ / ₄ "	6"
30°	S-2	1'-13/4"	10"	1'-81/4"	3½"	6½"	5½"	5¾"	6"
30	53	11½"	8½"	1'-5¾"	2"	7"	4¾"	4½"	71/4"
	<i>S4</i>	11½"	4"	1'-11/4"	21/4"	7"	3¾"	2½"	8½"
	<i>S</i> 1	1'-0"	8½"	1'-31/4"	5½"	7½"	6¼"	8½"	5½"
45°	<i>S-2</i>	1'-21/4"	8½"	1'-5½"	5¼"	6½"	71/4"	8½"	5½"
43	53	1'-0"	7"	1'-4"	3"	7½"	6¾"	7"	71/4"
	<i>S4</i>	1'-0"	3½"	11¾"	3¾"	7½"	5"	3½"	9"
	S1	1'-01/2"	6¼"	1 1 ¾"	7"	8"	6¾"	10¾"	4"
60°	S-2	1'-2 ³ / ₄ "	6¼"	1'-2"	7"	6¾"	8"	10¾"	4"
	53	1'-01/2"	5"	1'-1½"	4"	8"	8"	9"	7"
	54	1'-01/2"	2½"	9½"	5½"	8"	5½"	41/4"	91/4"

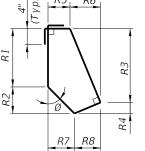


LAST

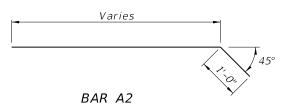
REVISION

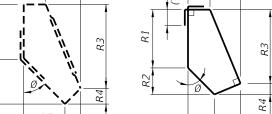
07/01/12





BARS 53 & 54

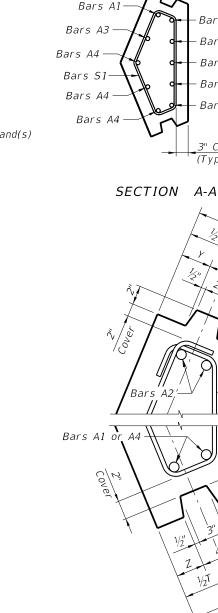




1 - PIECE 2 - PIECE BARS S1 & S2

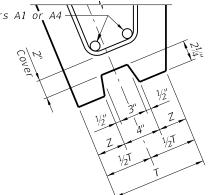
≥ DESCRIPTION:

FDOT 2014 DESIGN STANDARDS



See Detail "D" Bars A2 Bars A2 Bars A3 Bars A3 Bars A3 Bars A4 Bars A4 Bars A4 Bars A4 Bars S2 Bars A4 Bars A4 Bars A4 Bars A4 3" Cover See Detail "D" 3" Cover *(Typ.)* SECTION B-B

> SHEET PILE DIMENSIONS T (in.) 10 12 $3\frac{3}{16}$ $4\frac{3}{16}$ Y (in.) Z (in.) 4



(Typ.)

-Bars S

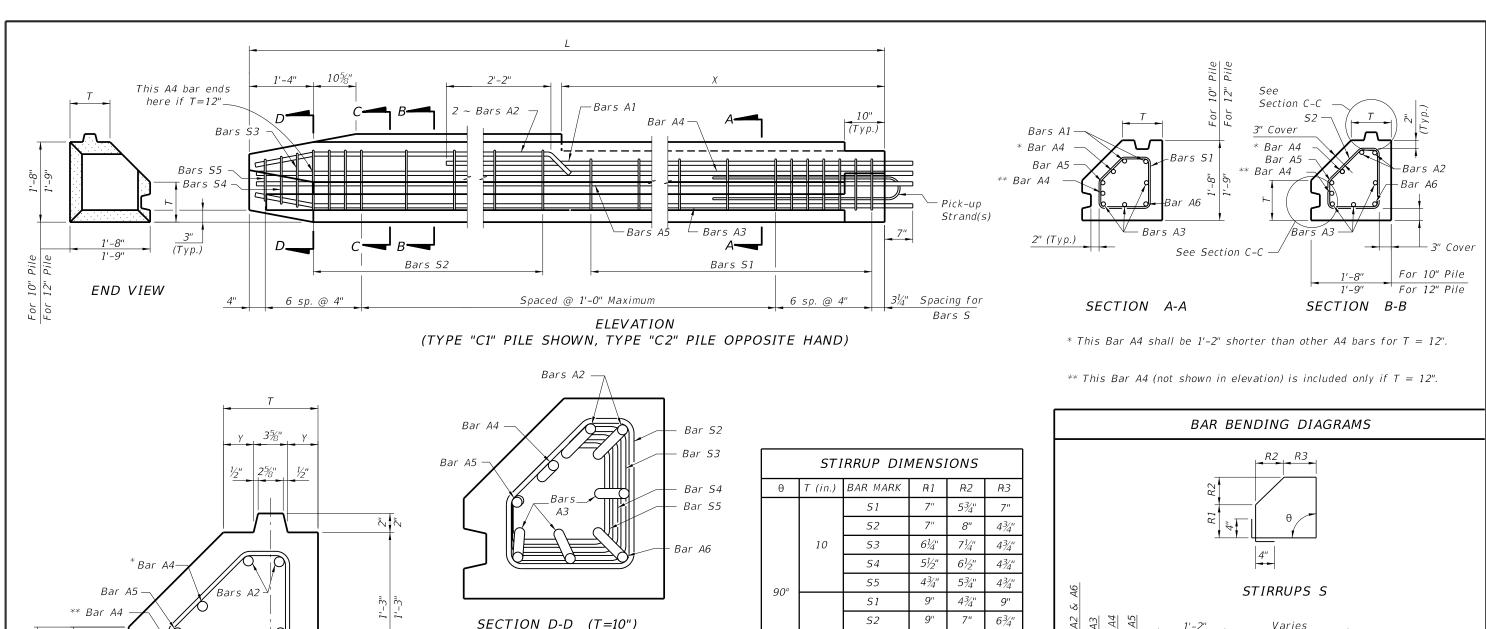
DETAIL "D" (TYPE "B1" PILE SHOWN, TYPE "B2" PILE OPPOSITE HAND)

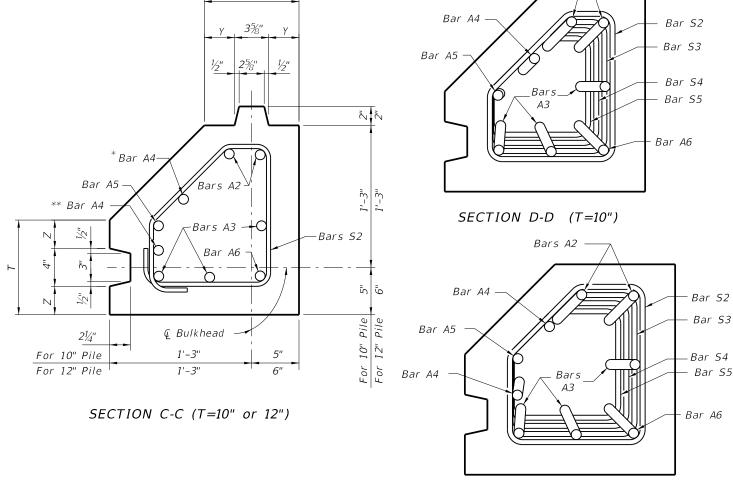
- 1. This drawing includes details for precast concrete corner piles for 10" and 12" thick sheet pile systems. The details apply equally to both thicknesses.
- 2. The bar configurations shown in Sections A-A and B-B shall be used for Ø angles between 15° and 75°. For Ø angles not shown, the reinforcing bar dimensions may be interpolated or extrapolated from the stirrup dimensions shown.
- 3. All bar dimensions are out-to-out.
- 4. Bars A are #8 and Bars S are #4.
- 5. Values for Stirrup Dimensions are shown for Ø equal to 30°, 45° & 60° only.
- 6. At the Contractor's option Bars S may be fabricated as a 2 piece bar with a minimum lap length of 1'-6", as shown in Bar Bending Diagrams.
- 7. If Type "B1" or "B2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3 & S4 in shop drawings.
- 8. If tongue must be on the opposite side from that shown all dimensions and Bars A, S2, S3 and S4 will be the same but opposite hand.
- 9. For Dimensions L, X and Angle Ø, see Sheet Pile Data Table in Structures Plans.

TYPE "B1" AND "B2" - VARIABLE ANGLE CORNER PILE

INDEX NO. 6040

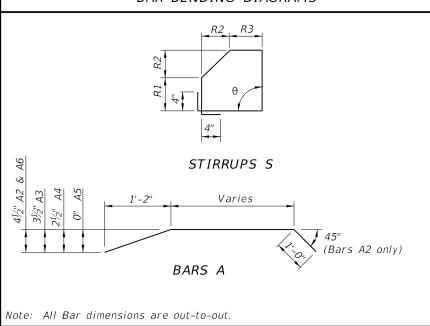
SHEET NO. 3 of 4





STIRRUP DIMENSIONS						
θ	T (in.)	BAR MARK	R1	R·2	R:3	
		S1	7"	5 ³ / ₄ "	7"	
		52	7"	8"	43/4"	
	10	53	61/4"	71/4"	4¾"	
		54		6½"	43/4"	
90°		<i>S5</i>	43/4"	5¾"	43/4"	
90		S1		43/4"	9"	
		52	9"	7"	6¾"	
		53	81/4"	$6\frac{1}{4}$ "	6¾"	
		54	7½"	5½"	$6\frac{3}{4}$ "	
		<i>S5</i>	6¾"	43/4"	6¾"	

SHEET PI	LE DIME	NSIONS
T (in.)	10	12
Y (in.)	3 ³ / ₁₆	4¾ ₆
Z (in.)	3	4



NOTES:

- 1. All bar dimensions are out-to-out.
- 2. Bars A are #8 and Bars S are #4.
- 3. This drawing includes information for precast Corner Piles for 10" and 12" thick Sheet Pile systems. The details apply to both thicknesses but the bar configurations change slightly according to the thickness values used.
- 4. If Type "C1" or "C2" pile is used as a Starter Pile show tongue on both sides of pile from Dim. X down. Show dimensions for Bars S2, S3, S4 & S5 in shop drawings.
- 5. If tongue must be on opposite side (Groove Side) from that shown, all dimensions and reinforcement shall follow the corresponding Tongue
- 6. For Dimensions L and X see Sheet Pile Data Table in Structures Plans.

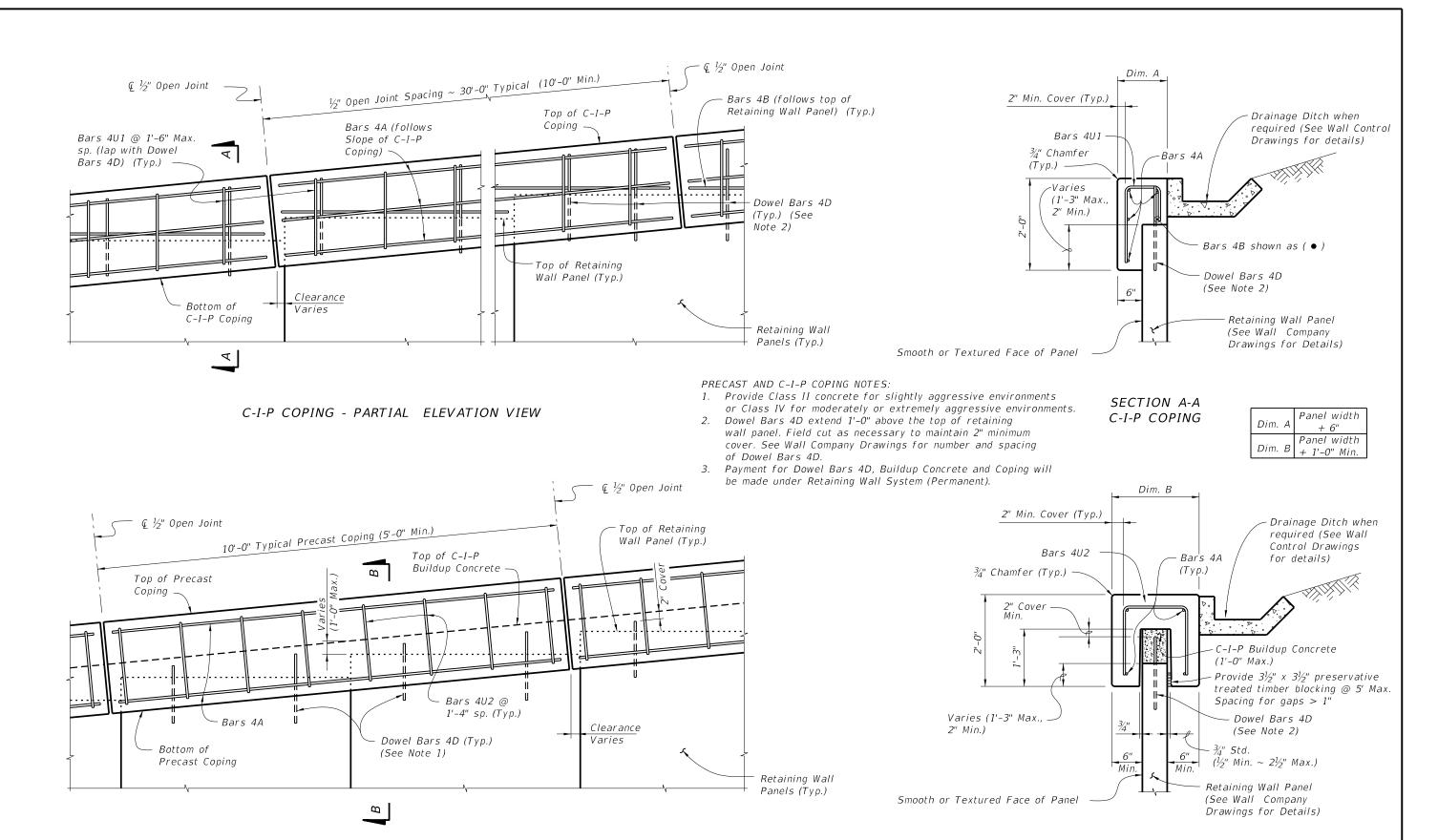
TYPE "C1" AND "C2" - RIGHT ANGLE CORNER PILE

≥ DESCRIPTION: LAST REVISION 07/01/12



FDOT 2014 DESIGN STANDARDS

SECTION D-D (T=12")

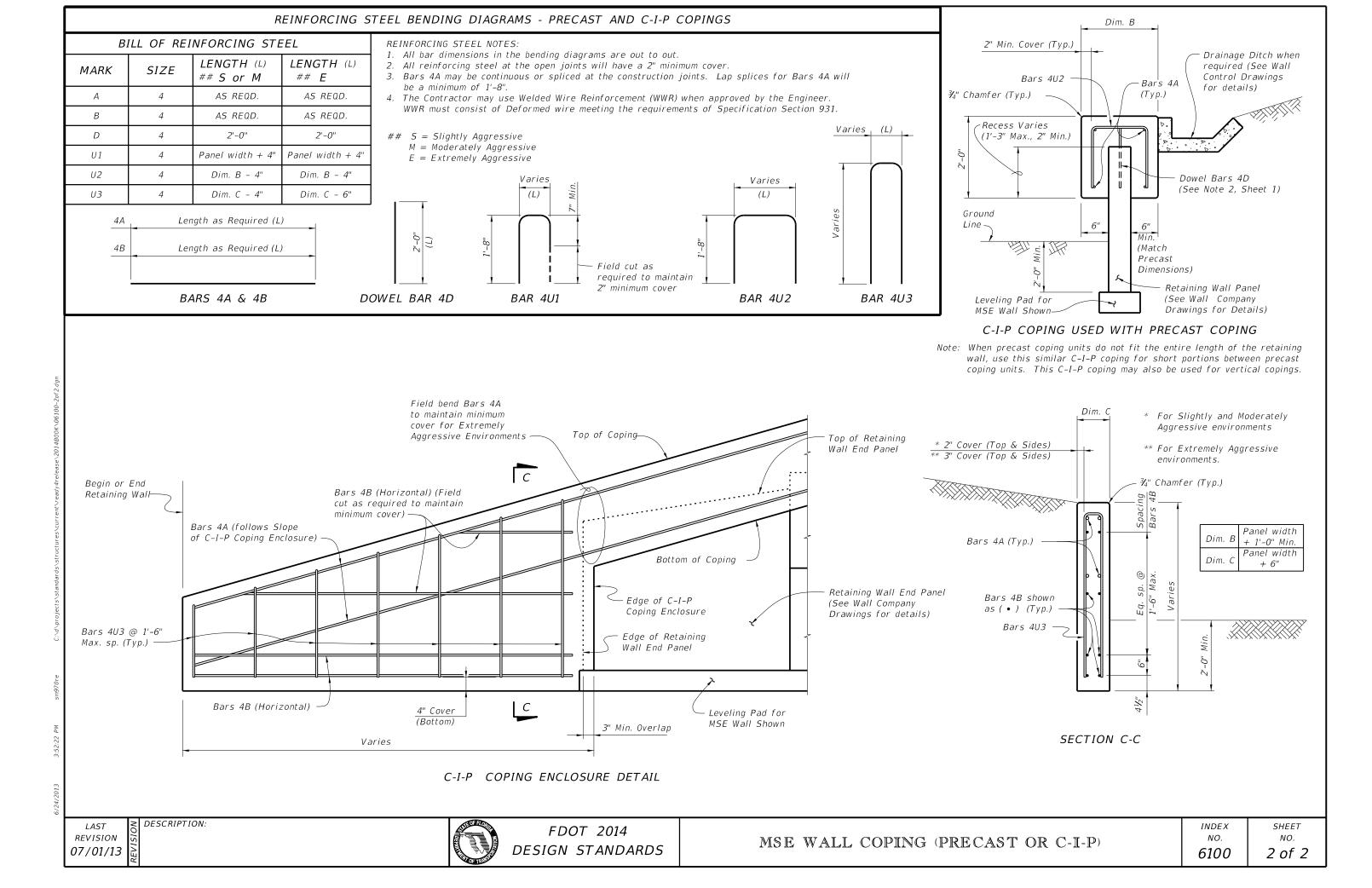


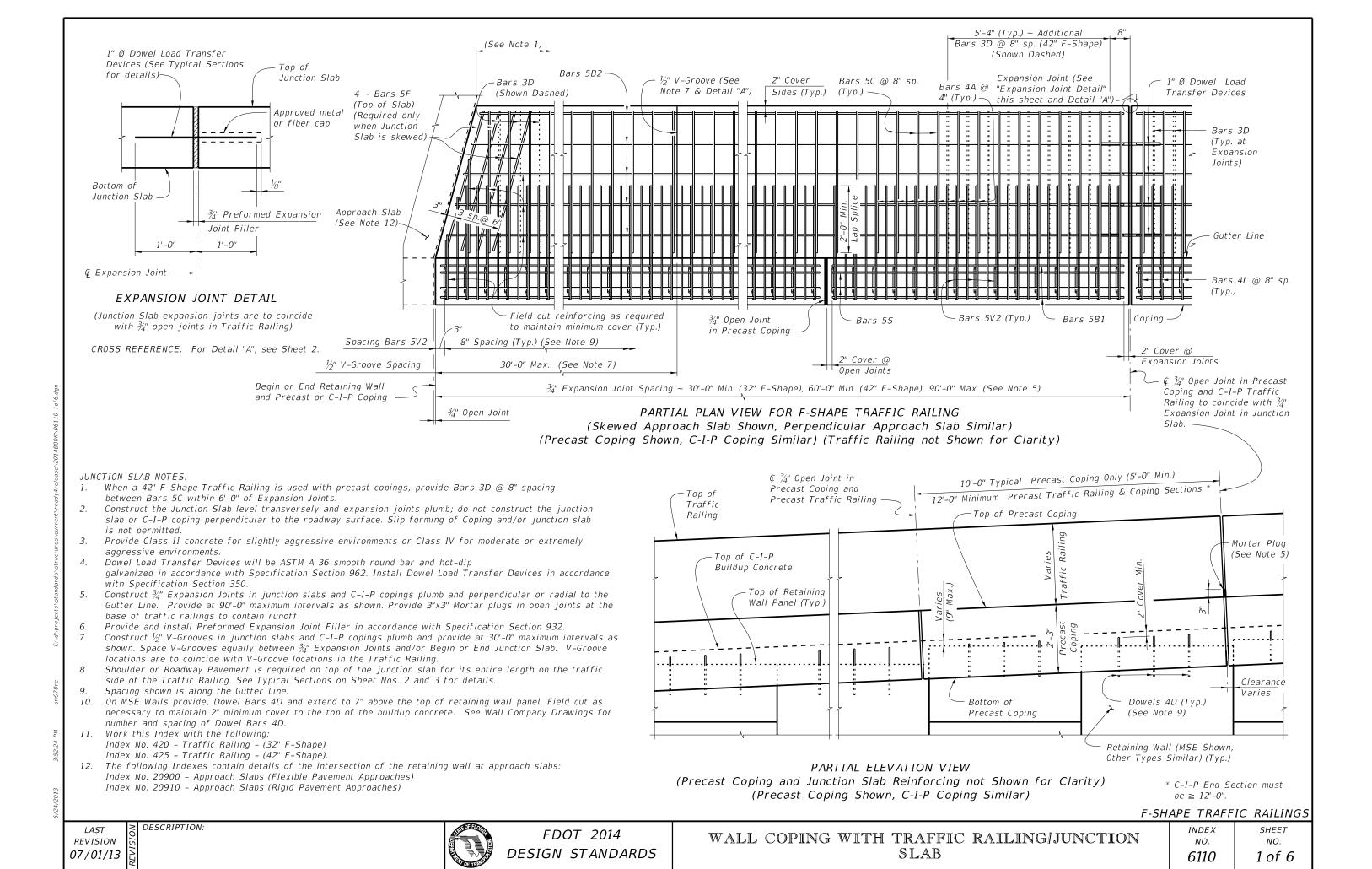
PRECAST COPING - PARTIAL ELEVATION VIEW

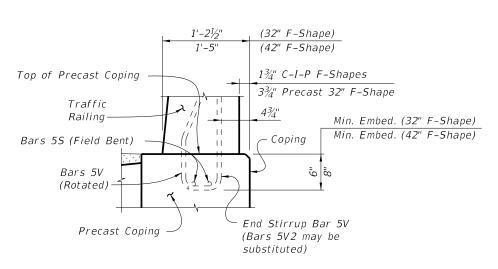
SECTION B-B PRECAST COPING

LAST OF DESCRIPTION:
REVISION OF DESCRIPTION:







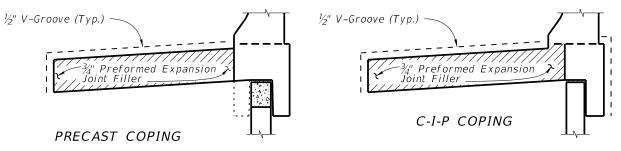


PARTIAL END VIEW OF TRAFFIC RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V and Bars 5S) (Precast Coping Shown, C-I-P Coping Similar)

NOTE: See Index No. 420 and Index No. 425, Detail "A" for details.

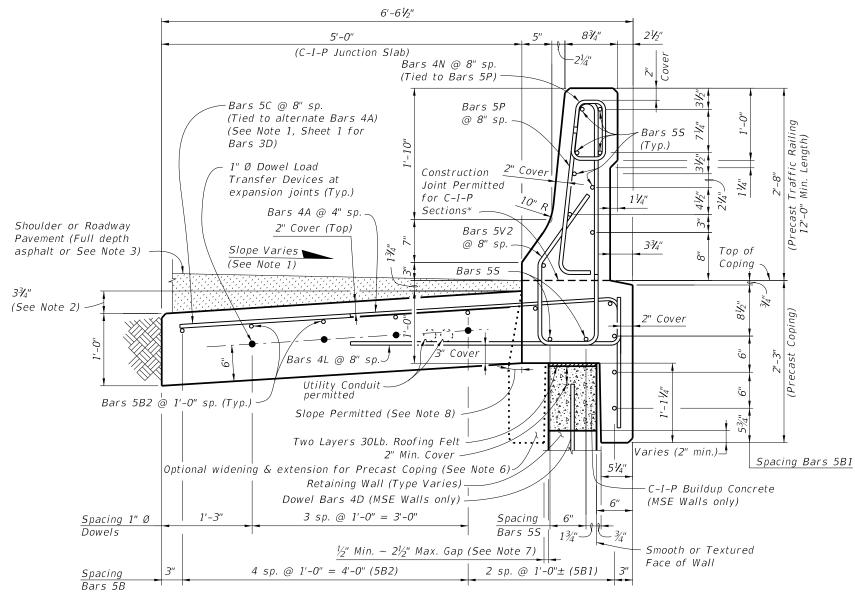
ESTIMATED QUANTITIES FOR PRECAST COPING				
ITEM	UNIT	QUANTITY		
Concrete (Precast Coping Only)	CY/LF	0.083		
Concrete (Precast Barrier & Coping)	CY/LF	0.169		
Concrete (C-I-P Junction Slab)	CY/LF	0.185		
Reinforcing Steel (Precast Coping & Traffic Railing)	LB/LF	52.67		
Reinforcing Steel (C-I-P Junction Slab) (Typ.)	LB/LF	12.52		
Additional Reinf. @ Expansion Joints (Dowels)	LB	21.36		

(The above concrete quantities are based on a max. superelevation of 6.25% and a 32" F-Shape Traffic Railing.



DETAIL "A"

(Showing Locations of $\frac{1}{2}$ " V-Grooves and $\frac{3}{4}$ " Preformed Expansion Joint Filler)



TYPICAL SECTION THRU PRECAST* 32" F-SHAPE TRAFFIC RAILING AND COPING WITH C-I-P JUNCTION SLAB

* C-I-P Traffic Railing and Coping Sections using precast dimensions and reinforcement are permitted at End Sections, Drainage Inlets and Light Pole Pedestals if slip forming is not used.

NOTES:

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. The 3¾" dimension corresponds to a maximum superelevation of 6.25%. For steeper superelevations increase this dimension to match roadway superelevation.
- 3. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finish grade.
- 4. Minimum length of Junction Slab between expansion joints is 30'-0".
- 5. At the Contractor's option, mechanical couplers may be used to splice reinforcing. Complete details, including reinforcement lengths are required in the Shop Drawings. Provide mechanical couplers in accordance with Specification Section 415. Mechanical couplers shall develop 125% of the bar yield strength.
- 6. Contractor to maintain stability of precast coping/traffic railing prior to junction slab completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 2" minimum concrete cover.
- 7. When the air gap between the precast coping extension and retaining wall exceeds $2\frac{1}{2}$, fill gap with full depth Expanded Polystyrene to provide a maximum $2\frac{1}{2}$ air gap.
- 8. Angle varies ~ 0° min., 20° max.

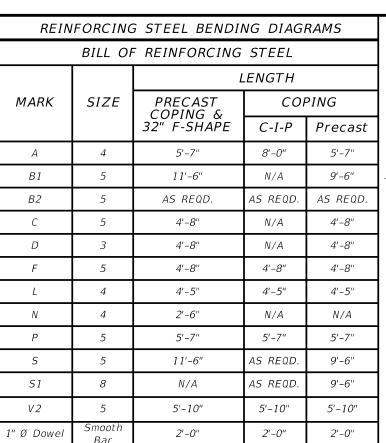
F-SHAPE TRAFFIC RAILINGS

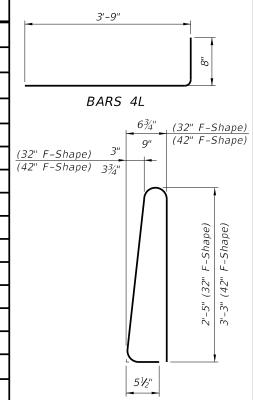
FDOT 2014 DESIGN STANDARDS WALL COPING WITH TRAFFIC RAILING/JUNCTION SLAB

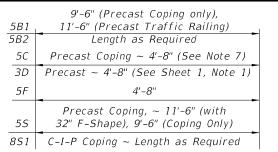
INDEX SHEET NO. 6110

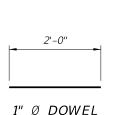
NO. 2 of 6

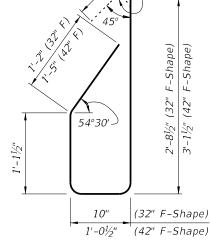
LAST REVISION 07/01/13 ∠ DESCRIPTION:







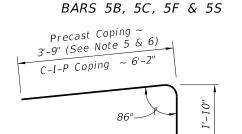


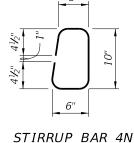


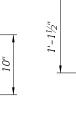
STIRRUP BAR 5P

Contractor's

option





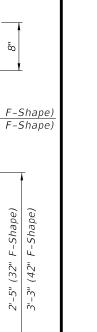


STIRRUP BAR 5V2

REINFORCING STEEL NOTES:

BAR 4A

- All bar dimensions in the bending diagrams are out to out.
- All reinforcing steel at expansion and open joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B & 5S will be a minimum of 2'-0".
- 4. For Precast Copings only, lap splice Bars 4A with Bars 5C. Lap splices will be a minimum of 2'-0".
- 5. The Contractor may use either full length Bars 4A or lap splice with Bars 5C at alternate Bars 4A for C-I-P Copinas.
- Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 1'-4\/'' (32" F-Shape) or 1'-7" (42" F-Shape).
- Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 4'-8".
- The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.
- 9. Contractor may use a single #5 stirrup in lieu of two bars for 5P and 5V2.



Dowels 4 sp. @ 1'-0'' = 4'-0''Spacing Bars 5B2 TYPICAL SECTION THRU C-I-P TRAFFIC RAILING WITH C-I-P JUNCTION SLAB AND C-I-P COPING (PRECAST COPING SIMILAR WITH C-I-P BUILDUP)

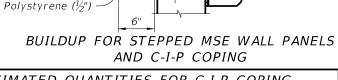
1'-3"

Bars 5B2 @ 1'-0" sp. (Typ.)

NOTES:

Spacing 1" Ø

- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. The $3\frac{3}{4}$ " dimension corresponds to a maximum superelevation of 6.25% For steeper superelevations increase this dimension to match roadway superelevation
- 3. For Rigid Pavement (Concrete), Junction Slab may be thickened to match finish grade.
- 4. Minimum length of Junction Slab between expansion joints is 30'-0" for 32" F-Shape or 60'-0" for 42" F-Shape.
- 5. See Index No. 420 & 425 for additional Traffic Railing Details.
- 6. Contractor to maintain stability of precast coping prior to junction slab completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 2" minimum concrete cover.
- 7. When the air gap between the precast coping extension and retaining wall exceeds $2\frac{1}{2}$, fill gap with full depth Expanded Polystyrene to provide a maximum $2\frac{1}{2}$ " air gap.
- 8. Angle varies ~ 0° min., 20° max.



ESTIMATED QUANTITIES FOR C-I-P COPING				
ITEM	UNIT	QUANTITY		
Concrete (Traffic Railing not Included)	CY/LF	0.268		
Reinforcing Steel (Typical) excluding Bars 5V2 and 5S (Typ.)	LB/LF	30.89		
Additional Reinf. @ Expansion Joint (Dowels)	LB/LF	21.36		

(The above concrete quantities are based on a max. superelevation of 6.25%, beneath a 32" F-Shape Traffic Railing on an MSE Wall).

F-SHAPE TRAFFIC RAILINGS

∠ DESCRIPTION: REVISION 07/01/13

FDOT 2014 **DESIGN STANDARDS**

WALL COPING WITH TRAFFIC RAILING/JUNCTION

 $6'-6\frac{1}{2}''$ (32" F-Shape)

6'-9" (42" F-Shape)

Bars 5P @ 8" sp.

Construction . 10"

Joint Regd.

Bars 5V2 @ 8" sp.

Bars 5S

See Note 8-

3" Cover

-Bars 4L @ 8" sp.

 $\frac{1}{2}$ " Min. ~ $2\frac{1}{2}$ " Max. Gap (See Note 7)

Two layers 30Lb. Roofing Felt

Retaining Wall (Type Varies)

Expanded Polystyrene (1/2" Side) (for C-I-P only)

3 sp. @ 1'-0'' = 3'-0''

3" Cover

5'-0"

(C-I-P Junction Slab)

Shoulder or Roadway

Pavement (Full depth

Sheet 1 for Bars 3D)

Transfer Devices at

expansion joints (Typ.)

Slope Varies

(See Note 1)

-1" Ø Dowel Load

asphalt or See Note 3)

-Bars 4A or for Precast Coping

Bars 5C @ 8" sp. (See Note 1,

Bars 4A @ 4" sp.

2" Cover (Top)

Optional widening & extension for Precast Coping (See Note 6)

21/4"

31/4"

6"

Expanded

Spacing |

2 sp. @ 1'-0"±

Bars 5S

10¾"

1'-01/4'

1/3" (32" F-Shape)

(42" F-Shape)

81/

31/4"

-Bars 5S

2" Cover

5½" (C-I-P)

 $5\frac{1}{4}$ " (Precast)

Face of Wall

Smooth or Texture

or 851 (Typ.)

Spacing Bars 5S (32" F-Shape)

Spacing Bars 8S1 (42" F-Shape)

(32" (42"

Top of

Coping

Spacing Bars

5B1 ~ Precast,

Optional Keyway

5B2 ~ C-I-P Copina

-Construction Joint Permitted

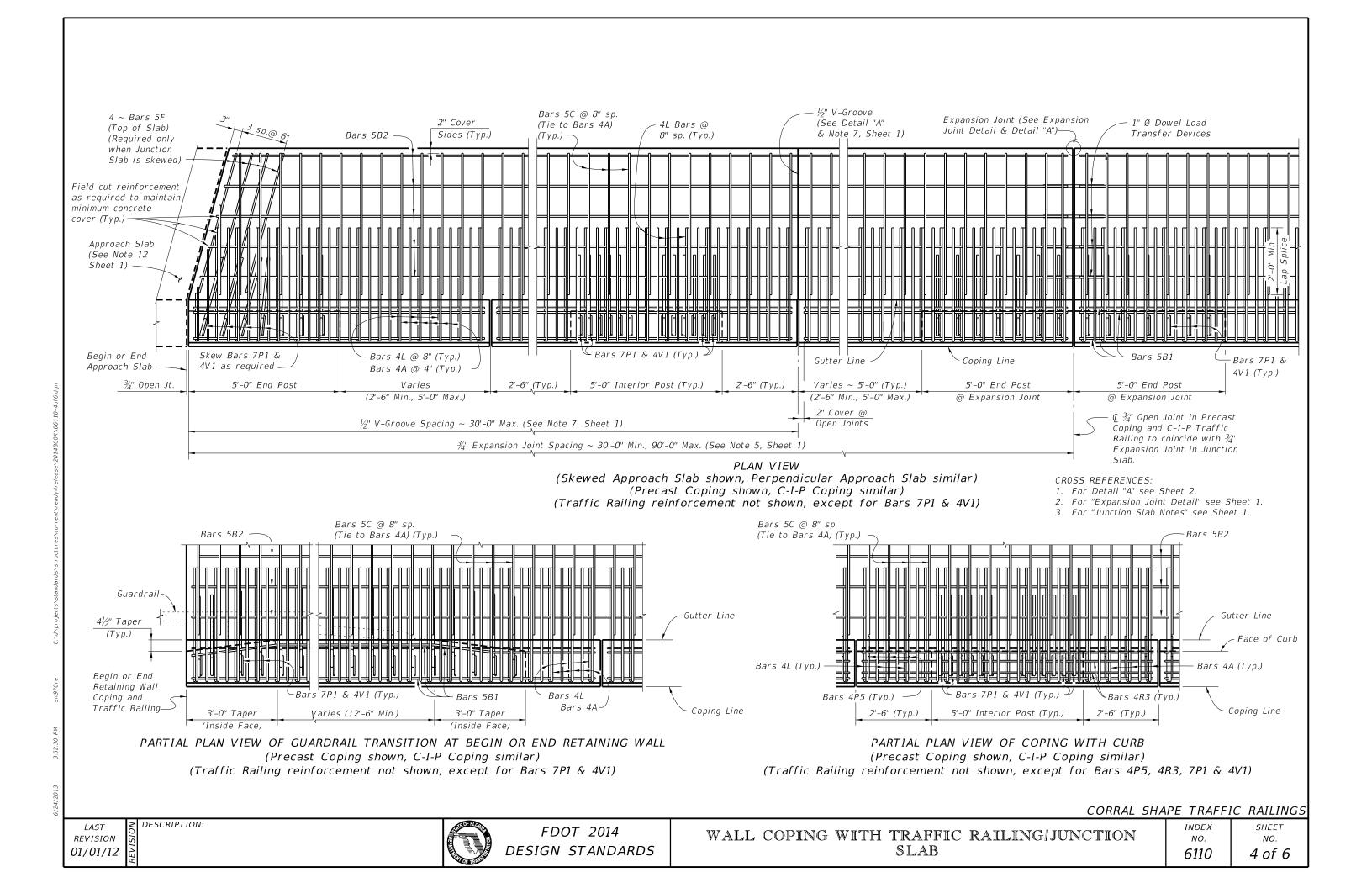
-Buildup for

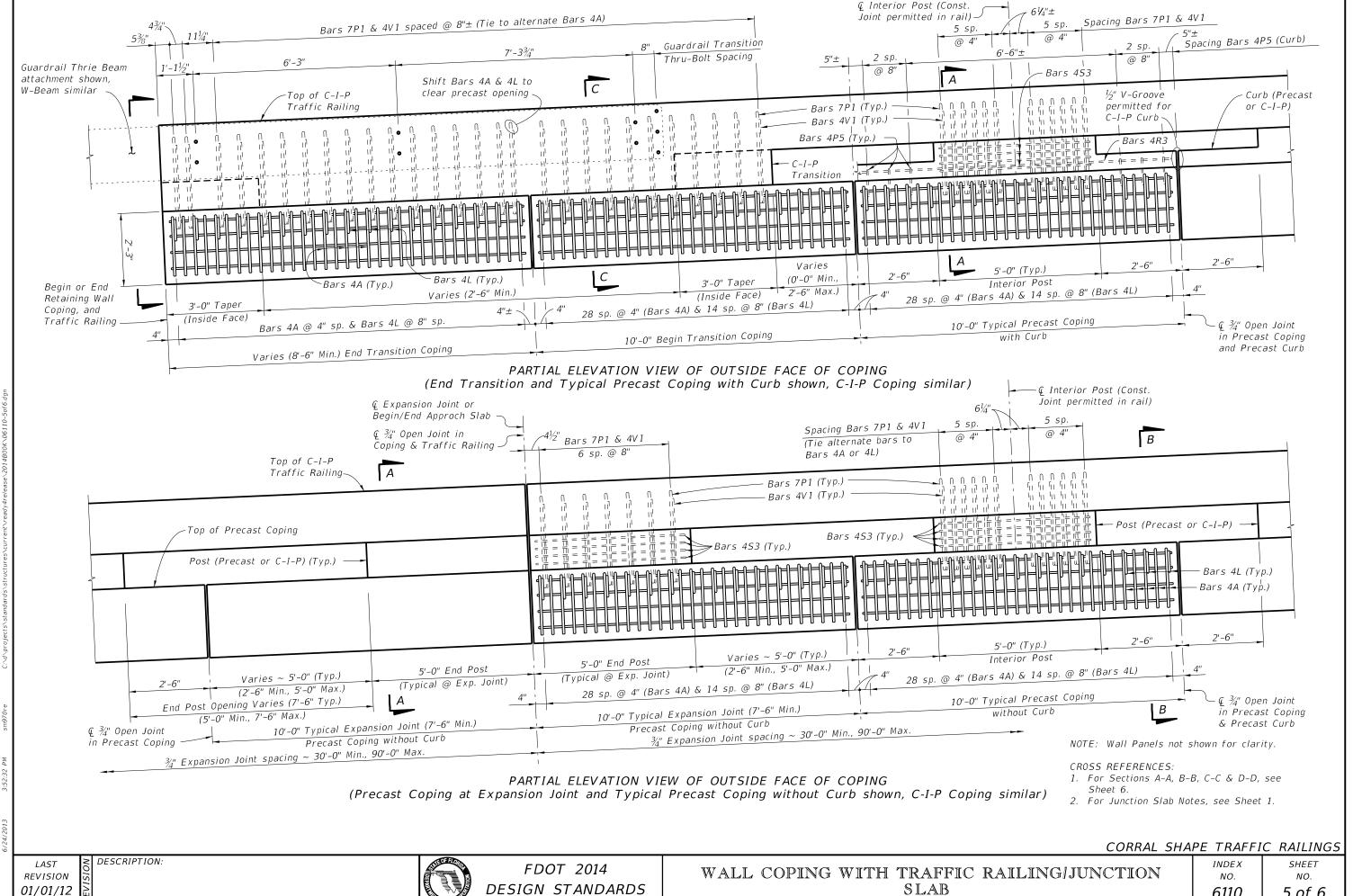
(0" min., $11\frac{1}{4}$ " max.)

stepped MSE Wall Panels

Coping

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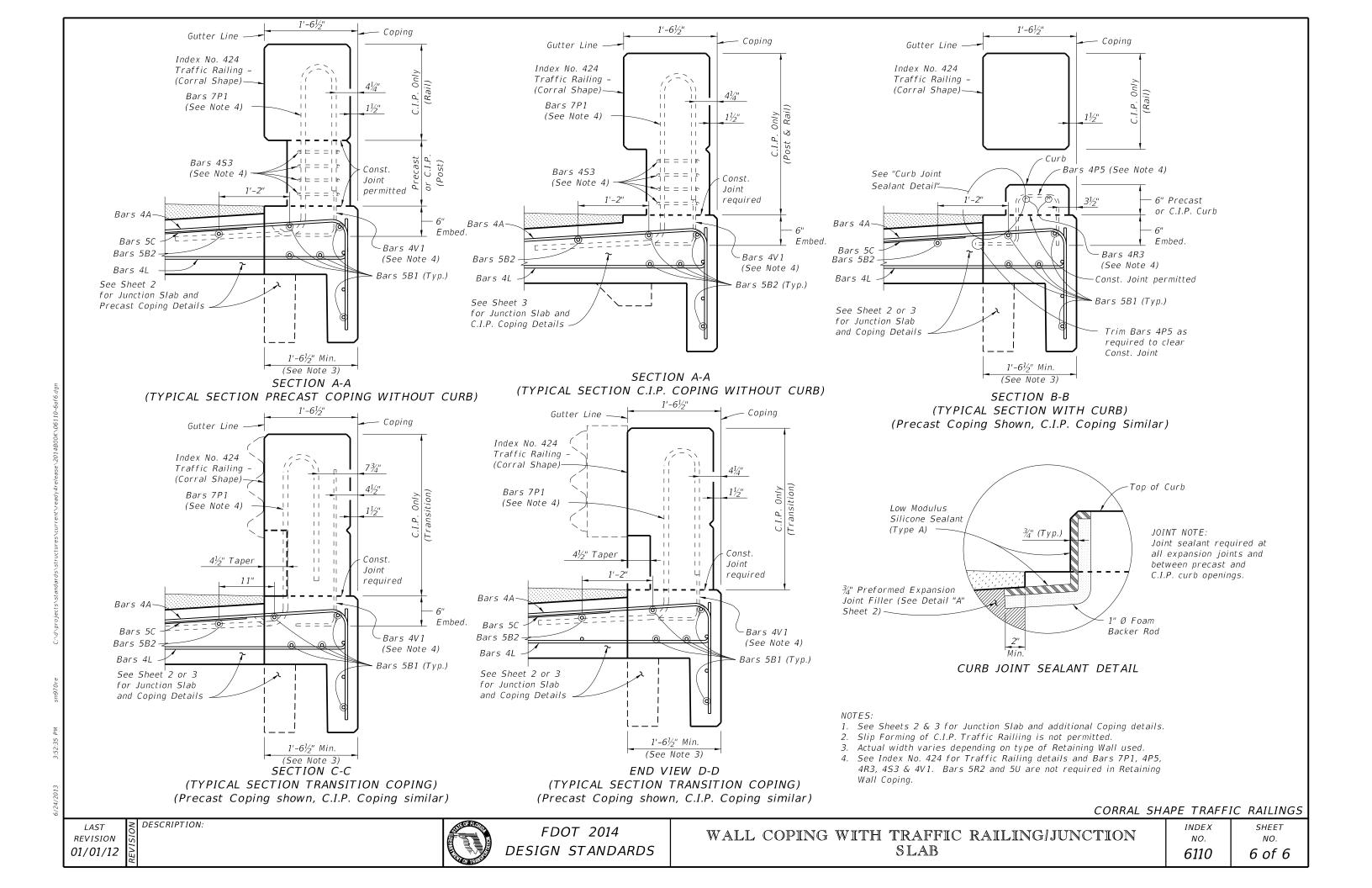


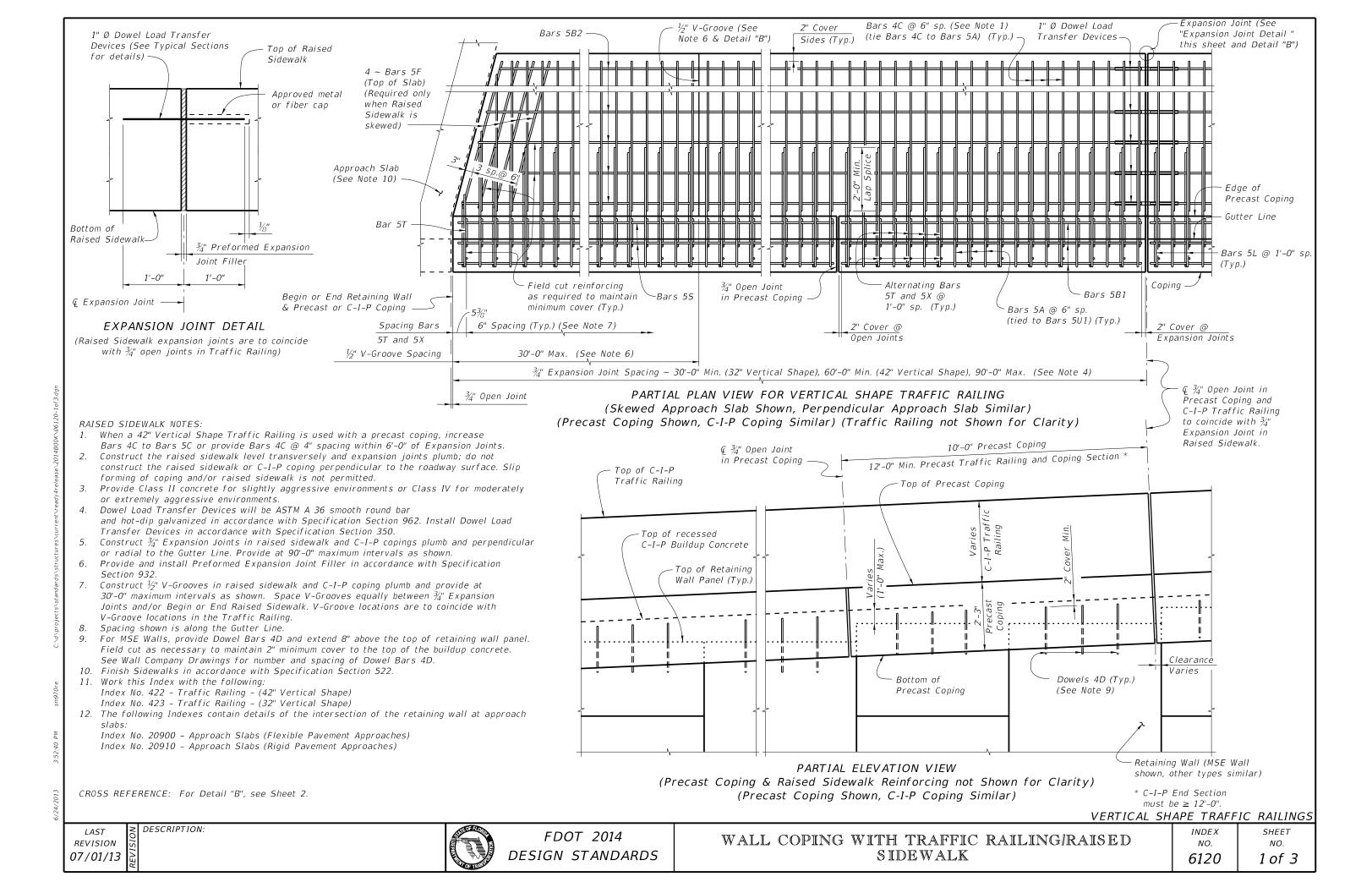
DESIGN STANDARDS

SLAB

6110

5 of 6



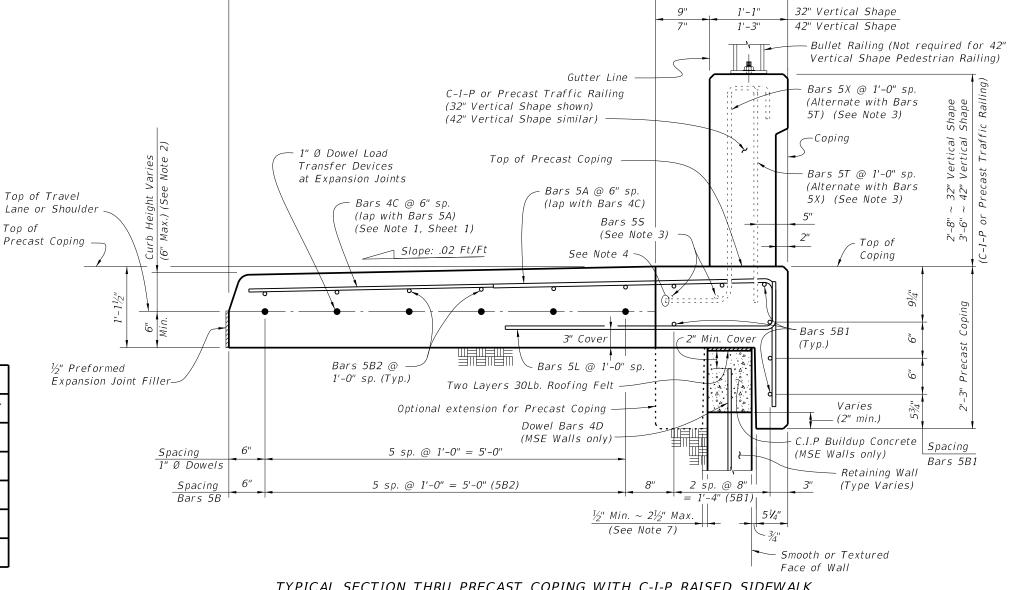


PARTIAL END VIEW OF TRAFFIC RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5S, Bars 5T and Bars 5X) (Precast Coping Shown, C-I-P Coping Similar)

NOTE: See Index No. 422 and Index No. 423, Railing End Detail for details.

ESTIMATED QUANTITIES FOR PRECAST COPING				
ITEM	UNIT	QUANTITY		
Concrete (Precast Coping)	CY/LF	0.095		
Concrete (C-I-P Raised Sidewalk)	CY/LF	0.232		
Reinforcing Steel (Precast Coping) excluding Bars 5T, 5X and 5S (Typ.)	LB/LF	23.90		
Reinforcing Steel (C-I-P Raised Sidewalk) (Typ.)	LB/LF	13.50		
Additional Reinf. @ Expansion Joints (Dowels)	LB	32.04		

The above concrete quantities are based on a Type D Concrete Curb (See Note 2).



7'-9"

1'-10"

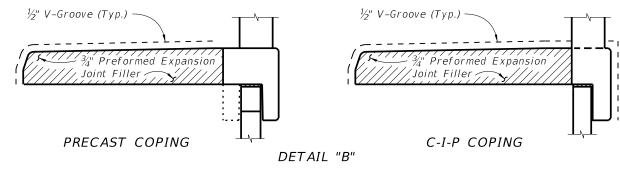
(Precast Coping)

5'-11" (C-I-P Raised Sidewalk) (See Note 2)

TYPICAL SECTION THRU PRECAST COPING WITH C-I-P RAISED SIDEWALK
AND RETAINING WALL AT EXPANSION JOINTS

NOTES:

- 1. Actual width varies depending on type of Retaining Wall used.
- 2. Match roadway curb shape (Type) and height. See Roadway Plans and Index No. 300. 5'-11" dimension is based on a 32" Vertical Shape Traffic Railing with a Type D curb adjacent to a 6'-0" wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.
- 3. See Index No. 422 and Index No. 423 for Bars 55, 5T & 5X and Bullet Railing details. Adjust vertical dimension of Bars 5T and 5X, see Reinforcing Steel Note 5.
- 4. Trim end of Bars 5T and 5X to clear construction joint for 42" Vertical Shape Traffic Railing.
- 5. At the Contractor's option, mechanical couplers may be used to splice reinforcing. Complete details, including reinforcement lengths are required in the Shop Drawings. Mechanical couplers shall develop 125% of the bar yield strength.
- 6. Contractor to maintain stability of precast coping prior to junction slab completion.
- 7. When the air gap between the precast coping extension and retaining wall exceeds $2\frac{1}{2}$, fill gap with full depth Expanded Polystyrene to provide a maximum $2\frac{1}{2}$ air gap.



(Showing Locations of $\frac{1}{2}$ " V-Grooves and $\frac{3}{4}$ " Preformed Expansion Joint Filler)

VERTICAL SHAPE TRAFFIC RAILINGS

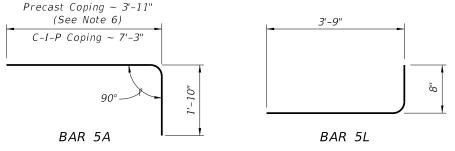
LAST REVISION 07/01/13



FDOT 2014 DESIGN STANDARDS WALL COPING WITH TRAFFIC RAILING/RAISED SIDEWALK

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1" Ø DOWEL

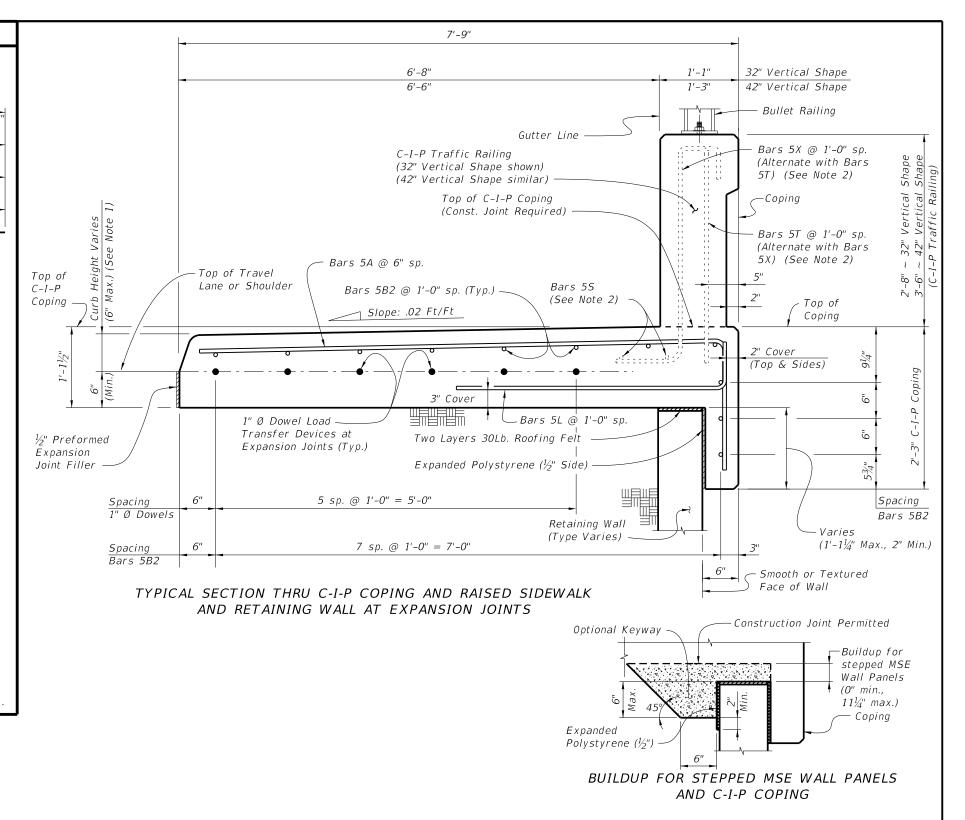


REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at expansion joints will have a 2" minimum cover.
- 3. Lap splices for Bars 5B will be a minimum of 2'-0".
- 4. Lap splice Bars 5A with Bars 4C. Lap splices will be a minimum of 2'-0".
- 5. See Index No. 422 and Index No. 423 for Bars 5S, 5T and 5X. Adjust vertical dimensions of Stirrup Bars 5T and 5X to 3'-0" for 32" Vertical Shape or 3'-10" for 42" Vertical Shape.
- 6. Dimension shown is for lap splice option. For mechanical coupler option, this dimension
- Dimension shown is for lap splice option. For mechanical coupler option, this dimension is 5'-8", and reinforcing size must be increased to #5 bars (Bars 5C).
- The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

ESTIMATED QUANTITIES FOR C-I-P COPING UNIT ITEM QUANTITY Concrete CY/LF 0.326 Reinforcing Steel (Typical) excluding LB/LF 35.03 Bars 5T, 5X and 5S (Typ.) Additional Reinf. @ Expansion Joints 32.04 (Dowels)

The above concrete quantities are based on a Type D Concrete Curb on a level Retaining Wall (See Note 1).



- 1. Match roadway curb shape (Type) and height. See Roadway Plans and Index No. 300. 6'-8" dimension is based on a 32" Vertical Shape Traffic Railing with a Type D curb adjacent to a 6'-0" wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.
- 2. See Index No. 422 and Index No. 423 for Bars 5S, 5T & 5X and Bullet Railing details. Adjust vertical dimension of Bars 5T and 5X, see Reinforcing Steel Note 5.

VERTICAL SHAPE TRAFFIC RAILINGS

∠ DESCRIPTION: LAST REVISION 07/01/13

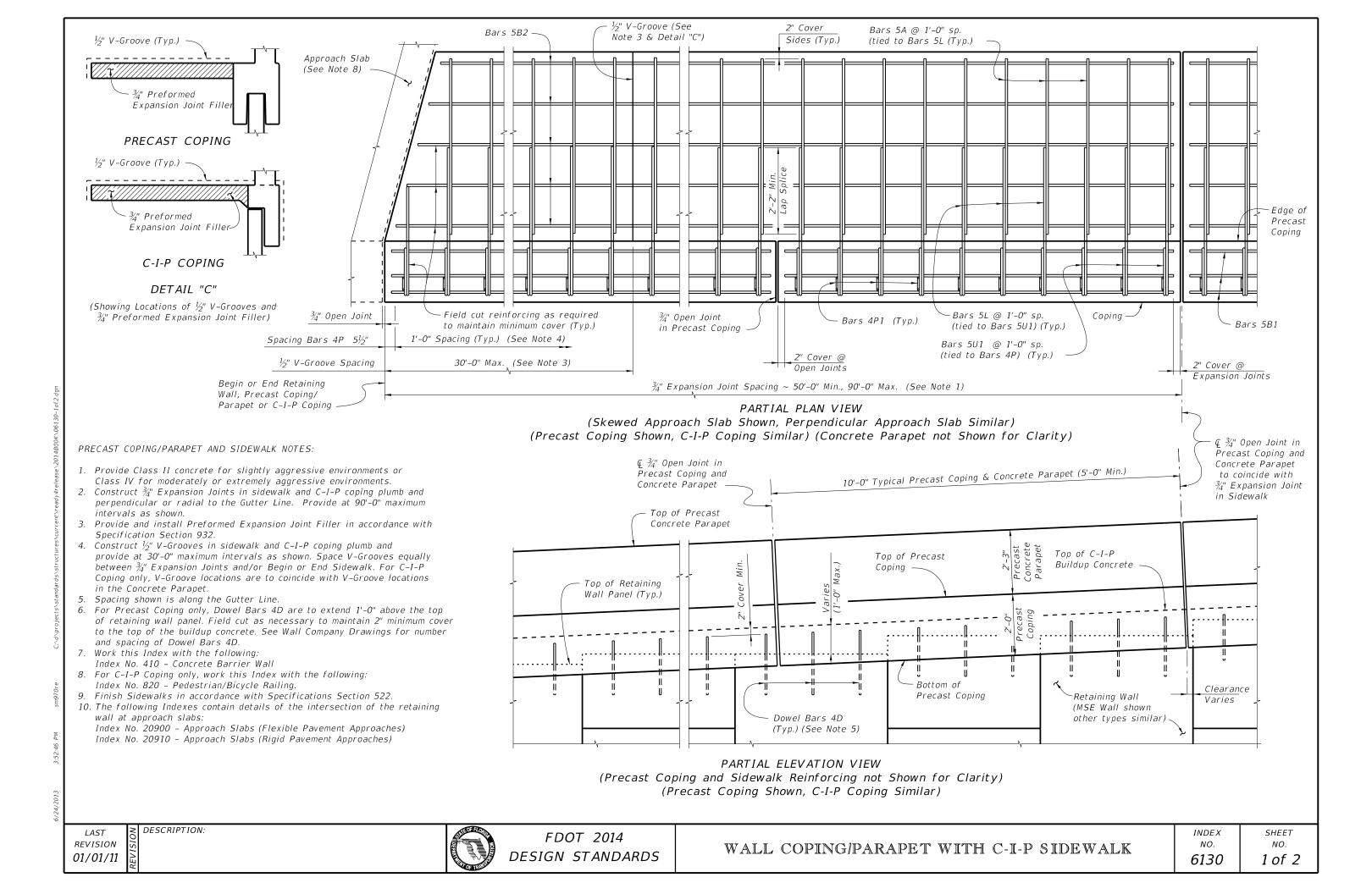


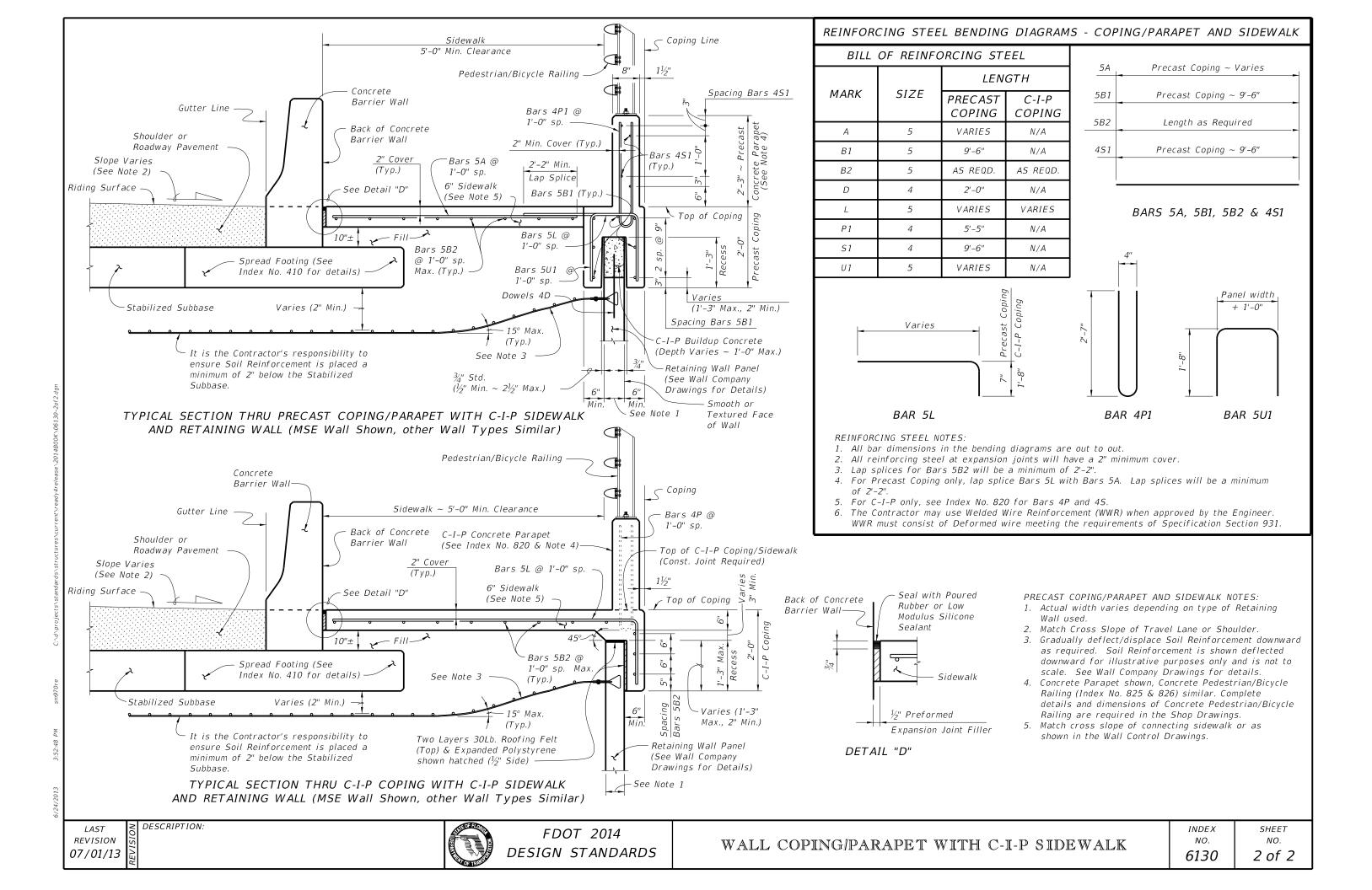
FDOT 2014 **DESIGN STANDARDS**

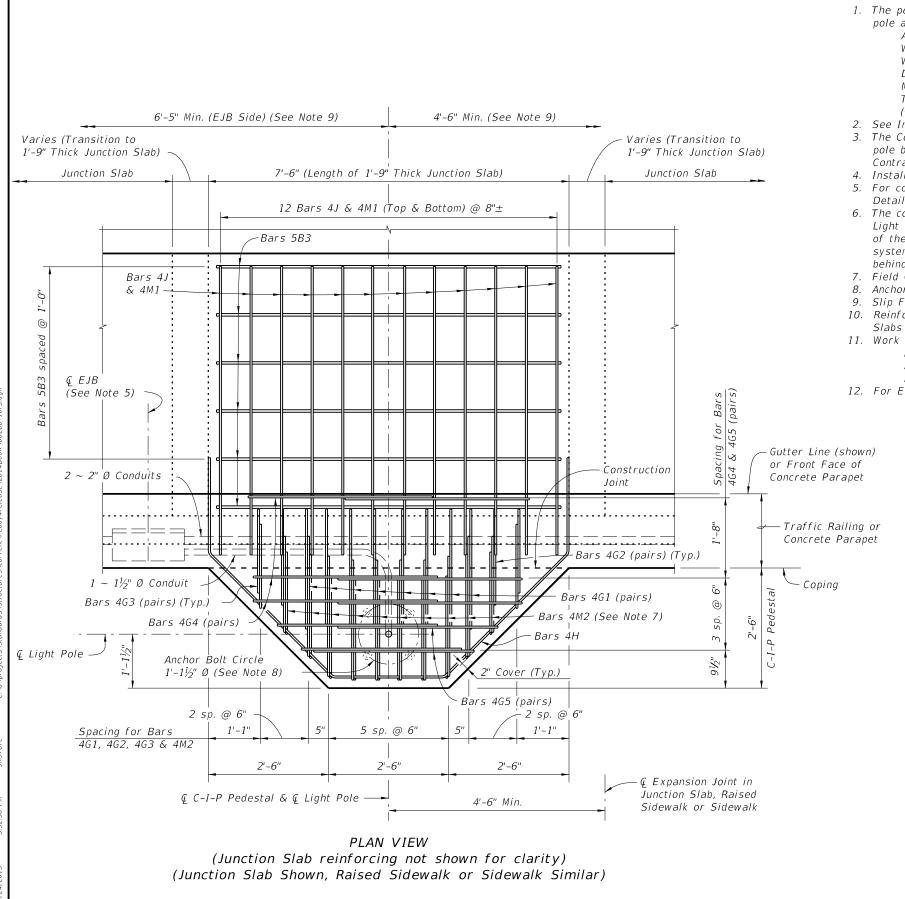
SIDEWALK

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WALL COPING WITH TRAFFIC RAILING/RAISED







≥ DESCRIPTION:

LAST

REVISION

07/01/13

LIGHT PEDESTAL NOTES:

1. The pedestal and junction slab are designed to resist the following working loads from the light pole applied at the top of the Pedestal:

Axial Deadload = 1.560 kipWind load Moment about Transverse Axis (*) = 40.60 kip-ftWind load Moment about Longitudinal Axis (*) = 28.30 kip-ft Dead load Moment about Longitudinal Axis (*) = 1.690 kip-ft Maximum Shear = 1.380 kip = 3.560 kip-ft Torsion about Pole Axis

(*) - Axis refers to Bridge Axis.

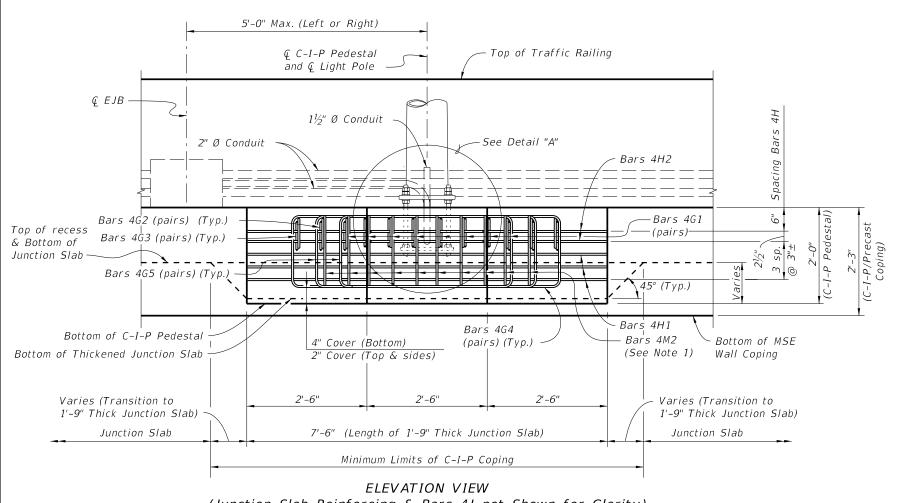
- 2. See Index No. 21200 for anchor bolt design and notes.
- 3. The Contractor is responsible for ensuring the anchor bolt design is compatible with the light pole base plate. Modifications to the anchor bolt design must be signed and sealed by the Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.
- 4. Install Anchor Bolts plumb.
- 5. For conduit, EJB and expansion/deflection fitting details, see Utility Conduit Detail Drawings.
- 6. The cost of anchor bolts, nuts, washers and anchor plates will be included in the Bid Price for Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, EJB and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Traffic Railing or Concrete Parapet that the pedestal is
- 7. Field Cut Bars 4M2 as required to maintain clearance.
- 8. Anchor Bolt pattern orientation will be as shown.
- 9. Slip Forming Method of construction requires the Engineer's approval within the limits shown.
- 10. Reinforcing shown for light pole pedestals is in addition to typical reinforcing for C-I-P Junction Slabs and Raised Sidewalks.
- 11. Work this Index with the following as appropriate:

Index No. 6110 Index No. 6120 Index No. 6130

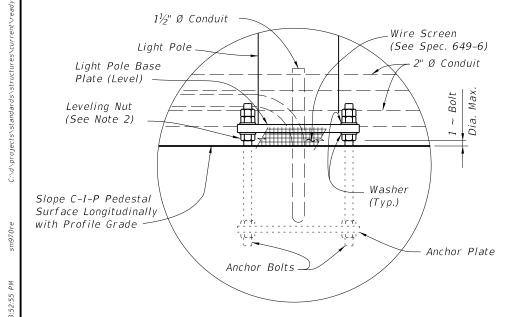
12. For Estimated Quantities, see Sheet No. 3.

6200

9'-01/2"



(Junction Slab Reinforcing & Bars 4J not Shown for Clarity) (Traffic Railing Shown, Concrete Parapet Similar) (Junction Slab Shown, Raised Sidewalk or Sidewalk Similar)



DETAIL "A"

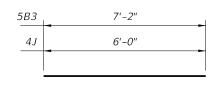
- 1. Field Cut Bars 4M2 as required to maintain minimum cover.
- 2. Maximum clearance between leveling nut and top of pedestal will not exceed anchor bolt diameter.

ESTIMATED QUANTITIES					
ITEM	UNIT	QUANTITY			
Concrete (Pedestal)	CY	0.926			
Concrete (Thickened Junction Slab)	CY	1.222			
Reinforcing Steel	LB	349			

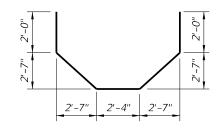
(The quantities above are for one C-I-P Light Pole Pedestal. The concrete quantity for the thickened junction slab is based on a 6" increase in thickness and a 5" wide retaining wall panel. Adjust thickened concrete quantity as required for raised sidewalks and sidewalks.)

REINFORCING STEEL BENDING DIAGRAMS - LIGHT POLE PEDESTAL

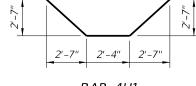
BILL OF REINFORCING STEEL							
MARK	SIZE	NO. REQD.	LENGTH				
В3	5	7	7'-2"				
G 1	4	16	5'-8"				
G2	4	4	4'-8"				
G3	4	4	4'-2"				
G4	4	6	8'-10"				
G5	4	4	7'-4"				
H1	4	3	9'-8"				
H2	4	2	13'-8"				
J	4	12	6'-0"				
M 1	4	12	5'-10"				
M2	4	10	3'-8"				



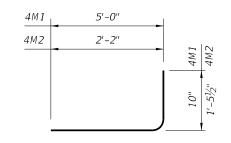
BARS 5B3 & 4J



BAR 4H2



BAR 4H1



BAR 4M1 & 4M2

REINFORCING STEEL NOTES:

2'-6" 2'-0"

1'-9"

3'-8"

2'-11"

BARS 4G1, 4G2, 4G3,

4G4 & 4G5

1. All bar dimensions in the bending diagrams are out to out.

4G1

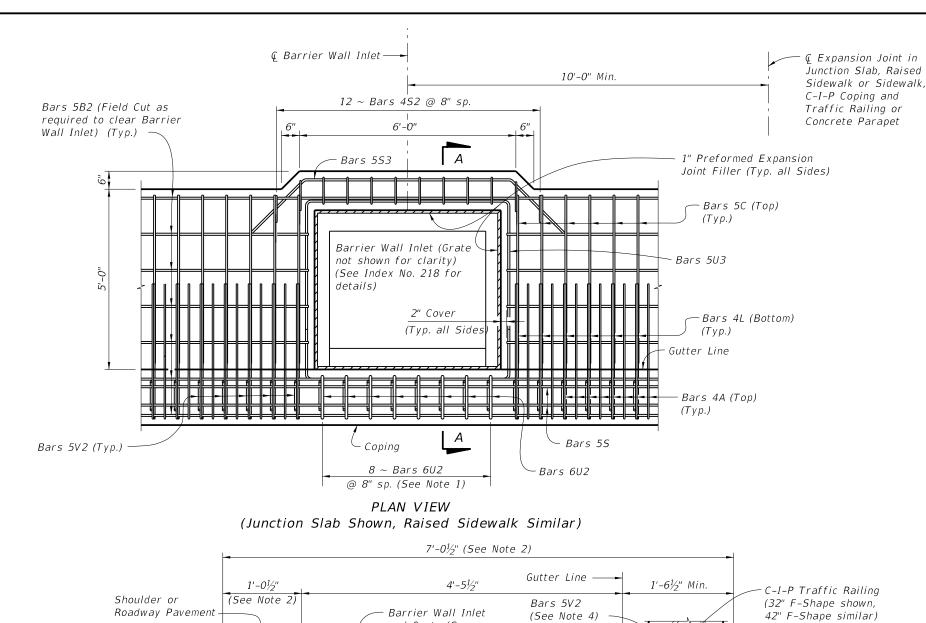
4G3

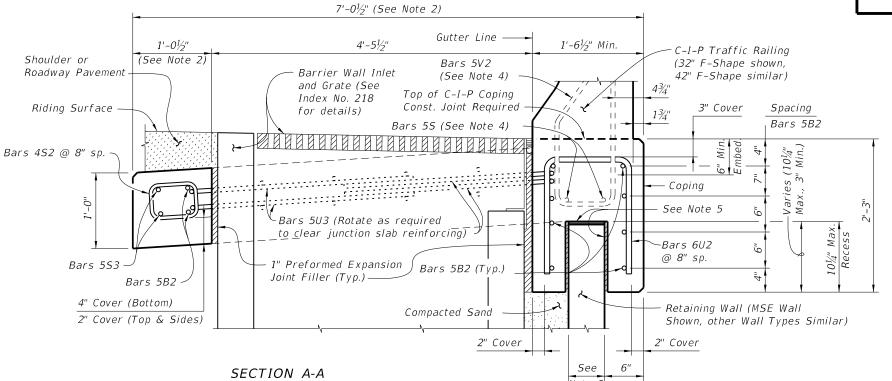
4G4

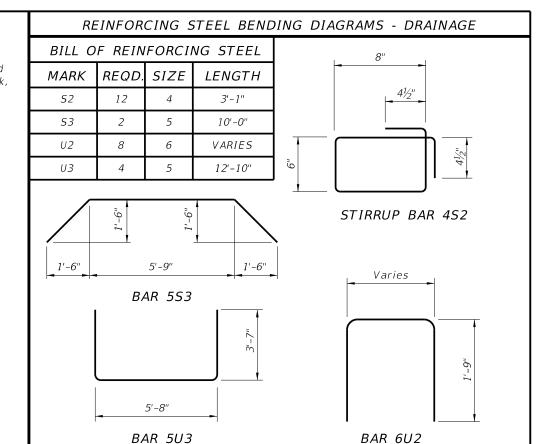
4G5

- 2. Lap splices for Bars 4G1, 4G2 & 4G3 will be a minimum of 1'-4". Lap splices for Bars 4G4 & 4G5 will be a minimum of 1'-8".
- 3. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of deformed wire meeting the requirements of Specification Section 931.

≥ DESCRIPTION:







REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at open joints will have a 2" minimum cover.
- 3. See Index Nos. 6110, 6120 & 6130 for Bars 4A (or 5A), 5B, 5C and 4L (or 5L).
- 4. The Contractor may use Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

NOTES:

- 1. Spacing shown is along the Gutter Line. Spacing shown is for C-I-P Junction Slab. For C-I-P Raised Sidewalks or Sidewalks, match bar spacing and size shown in Typical Sections (i.e., 11 ~ Bars 5U2 and 15 ~ Bars 452 @ 6" spacing for Raised Sidewalks).
- 2. Dimensions shown are for junction slab. Increase width as required for C-I-P Raised Sidewalk and Sidewalks.
- 3. Actual location & width vary depending on type of Retaining Wall used.
- 4. See Index No. 6110 for Bars 5V2 and 5S.
- 5. Two Layers 30Lb. Roofing Felt (Top) & Expanded Polystyrene shown hatched ($\frac{1}{9}$ " Each Side).
- 6. Locate © Barrier Wall Inlet a minimum of 10'-0" away from © Expansion Joints in Junctions Slab, Raised Sidewalk or Sidewalk, \overline{C} -I-P Coping and Traffic Railing or Concrete Parapet.
- 7. Work this Index with the following as appropriate: Index No. 6110

Index No. 6120

Index No. 6130

LAST REVISION 07/01/13 ∠ DESCRIPTION:



FDOT 2014 DESIGN STANDARDS

INDEX SHEET NO. NO. 6201 1 of 1

SECTION THRU JUNCTION SLAB, BARRIER WALL INLET AND RETAINING WALL (Junction Slab Shown Raised Sidewalk Similar)