01/01/12

- 1. The illustrations for guardrail applications are standard configurations; adjustments are to be made as required by site specific conditions to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figures 1 and 2, and other application details of this Index.
- 3. One Panel (i.e., panel length) equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standards (indexes) or specifically called for in the plans.

Post spacing shall be 6'-3" except that reduced spacing shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Detail J and Index No. 402) and transitions to redirective crash cushions, (b) the conditions in Note No. 7 below, (c) special post applications, (d) reduced post spacing required for specific end anchorage assemblies, and, (e) specific spacing called for in the plans.

- 4. Guardrail mounting height for the W-beam without rub rail and for thrie-beam is 1'-9" to the center of beam, and for W-beam with rub rail 2'-0" to center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a tolerance of 3" above and 1" below the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines). For guardrail placed on slopes beyond the shoulder point, there shall be no deviation more than 1" below to 3" above the desired height within any 25 foot section of guardrail.
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end treatments for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end treatments will be constructed only when restraints prevent construction of flared end anchorages.

Guardrail end anchorage assemblies shall be of the type called for in the plans. If the plans call for end anchorage assembly "flared" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDOT approved flared assembly provided in this Index or identified on the Qualified Products List (QPL), subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for end anchorage assembly "parallel" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDOT approved parallel assembly provided in this Index or identified on the QPL, subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for a specific end anchorage assembly, substitutions with other end anchorage assemblies will not be permitted unless approved by the Engineer. Approved substitutions will not be eligible for CSIP consideration.

When an end treatment is attached to guardrail with Pedestrian Safety Treatment, only end treatment systems with timber posts are to be used.

Existing approved proprietary end anchorage systems are identified on the Qualified Products List (QPL). After January 1, 2011 – manufacturers seeking approval of new proprietary end anchorage systems for inclusion on the QPL must submit application along with design documentation showing the end anchorage system; is crash tested to Test Level 3 criteria in accordance with the Manual for Assessing Safety Hardware (MASH), is accepted by FHWA for use as a guardrail end anchorage system, and is compatible with FDOT guardrail systems. System approvals will be contingent on FDOT's evaluation of crash test performance results for consistency with FDOT guardrail application and use. If approved, installation drawings signed and sealed by a professional engineer licensed in the State of Florida will be required.

- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 4' minimum for standard W-beam, other guardrail configurations with reduced post spacing may be applicable; see General Note No. 11 and the minimum offset table on Sheet 19. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier wall shall be used unless other shielding is approved by the Engineer of Record. See Index No. 410 for safety shape concrete barrier walls and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at roadside hazards or other areas where the Engineer has deemed guardrail necessary, guardrail should be considered on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone and fill heights are 6' or greater. Curbed sections where fill slopes are steeper than 1:3 and fill heights are 6' or greater within 22' of the traveled way should be evaluated for installation of guardrail. Additional guidance for evaluating the need for guardrail can be found in the Plans Preparation Manual.
- 9. The guardrail to bridge connections contained in this Index are for bridges with Test Level 4 traffic railing barriers. For guardrail to concrete barrier wall connections see Index No. 410. For existing bridges receiving retrofit traffic railing barriers see Index No. 402.

- 10. The W-beam guardrail system in this index is the standard system to be used on the State Highway System where a Test Level 3 semi-rigid barrier is required.
- 11. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic railing barriers, to concrete and certain water filled safety shaped barriers, certain crash cushions and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 402, 410 and 414. The use of thrie-beam guardrail with standard offset blocks (Test Level 3 semi-rigid system) may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
 - a. W-beam deflection is marginal,
 - b. W-beam with rub rail considered functionally deficient,
 - c. Vehicle overriding W-beam is probable,
 - d. Drainage will be impeded or blocked by the use of concrete barrier wall (subject to deflection space requirements),
 - e. High frequency of repairs to W-beam,
 - f. Spandrel beam with low deflection needed around unrelocatable structure,
 - g. Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g., passenger vans and small buses).

The modified thrie-beam guardrail is a Test Level 4 semi-rigid system and may be used where a Test Level 4 guardrail is required.

- 12. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
 - a. Wide medians where approach end anchor is located outside of opposing roadway clear zone,
 - b. Medians of uniform width that are occupied by other transportation and joint use facilities,
 - c. Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail installations,
 - d. Medians of bifurcated roadways.
- 13. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 14. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crashworthiness at terminals. Crash cushions shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 15. Corrugated sheet steel beams, end shoes, end sections and backup plates shall conform to the current requirements of AASHTO M180, Class A, Type II (zinc) coating. All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTO requirements.
- 16. Offset blocks:
 - a. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing steel offset blocks may remain throughout the service life of the existing guardrail. PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS are tabulated on Sheet 16.
 - b. Plastic offset block installations shall be constructed on guardrail outside of approach end anchorage assemblies or any transition system connecting to a rigid or thrie-beam barrier.
- 17. Where necessary to enlarge or add holes to galvanized guardrail, the work will be done by drilling or reaming. Damaged galvanized guardrail will be metalized in accordance with Sections 562 and 971 of the Standard Specifications. No burning of holes will be permitted.
- 18. For guardrail reflector details see Sheet 17.
- 19. Any run of guardrail with existing concrete posts that is being reset under a construction or maintenance contract shall be reset using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 20. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for CSIP consideration.
- 21. On roadways designated for reverse laning, all downstream ends of guardrail that are not shielded or that are not designed as approach end terminals shall be marked with post-mounted Type 3 Object Markers. Trailing bridge ends and trailing shoulder concrete barrier wall ends shall be marked with Type 3 Object Markers except where there is trailing end guardrail. Object markers to be installed facing reverse laning traffic. The cost of the object marker shall be included in the cost of the guardrail.

SHFFT

REVISION

01/01/12

GENERAL NOTES

1. These drawings are representative of the various proprietary guardrail end anchorage assemblies listed on the Department's Qualified Products List (QPL). For specific details and requirements see the vendor drawings on the QPL at

www.dot.state.fl.us/specificationsoffice/

- 2. These drawings present the general graphics to show the limits of payment for guardrail and end anchorage assemblies, modifications to the shoulder and placement of the miscellaneous asphalt mow strips.
- 3. These drawings, along with the various vendor drawings on the QPL, are intended to show sufficient details for installation of the end anchorage assemblies and their connection to shoulder guardrail. This precludes the requirement for shop drawing submittals unless otherwise called for in the plans. The various end anchorage assemblies shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.
- 4. The various proprietary end treatments listed on the QPL are intended for use as approach end guardrail anchorages for shoulder guardrail. The effective length of the end treatments vary-refer to the vendor drawings on the QPL for the length and the use of special panels and details. Standard guardrail, guardrail transitions or other special treatments shall not be included within the limits of the end anchorage assembly. See the vendor drawings for the alignment of the end treatment with respect to the normal guardrail alignment.
- 5. Flared or parallel end anchorage assemblies shall not be used in medians where horizontal clearance requires use of a back rail.
- 6. Each of the various end anchorage assemblies have unique features. Careful attention should be given to the types and orientation of the posts and other components. Refer to the vendor drawings on the QPL for the specific requirements of each system.
- 7. For galvanizing requirements of the metallic components see Standard Specifications Section 967.
- 8. Test Level 3 crash tested end anchorage assemblies shown on the QPL are suitable for all design speeds.
- 9. Flared end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Flared), EA.

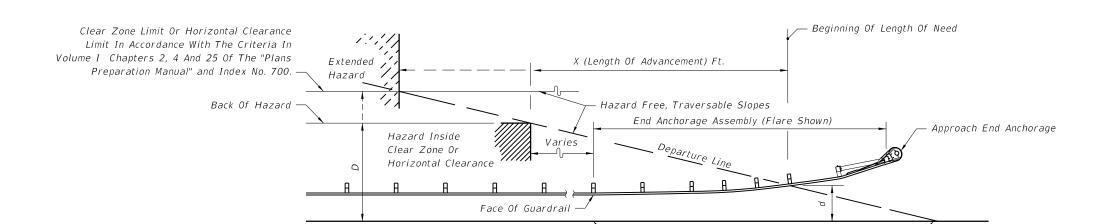
Parallel end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Parallel), EA.

The unit price for end anchorage assemblies shall be full compensation for furnishing and installing all components in accordance with the plans, the manufacturer's detail drawings, procedures and specifications and these drawings.

APPROACH END ANCHORAGE DETAILS

FDOT DESIGN STANDARDS
2013





Design Speed mph	X (Length Of Advancement) Ft. (See NOTES 1 & 2)
≤ 45	= 16 (D-d)
≥ 50	= 13 (D-d)

└─ Edge Of Traffic Lane

Point Of Departure

NOTES

- 1. Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.
- 2. The flared end anchorage with 4' nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

Equation Variables:

D = Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

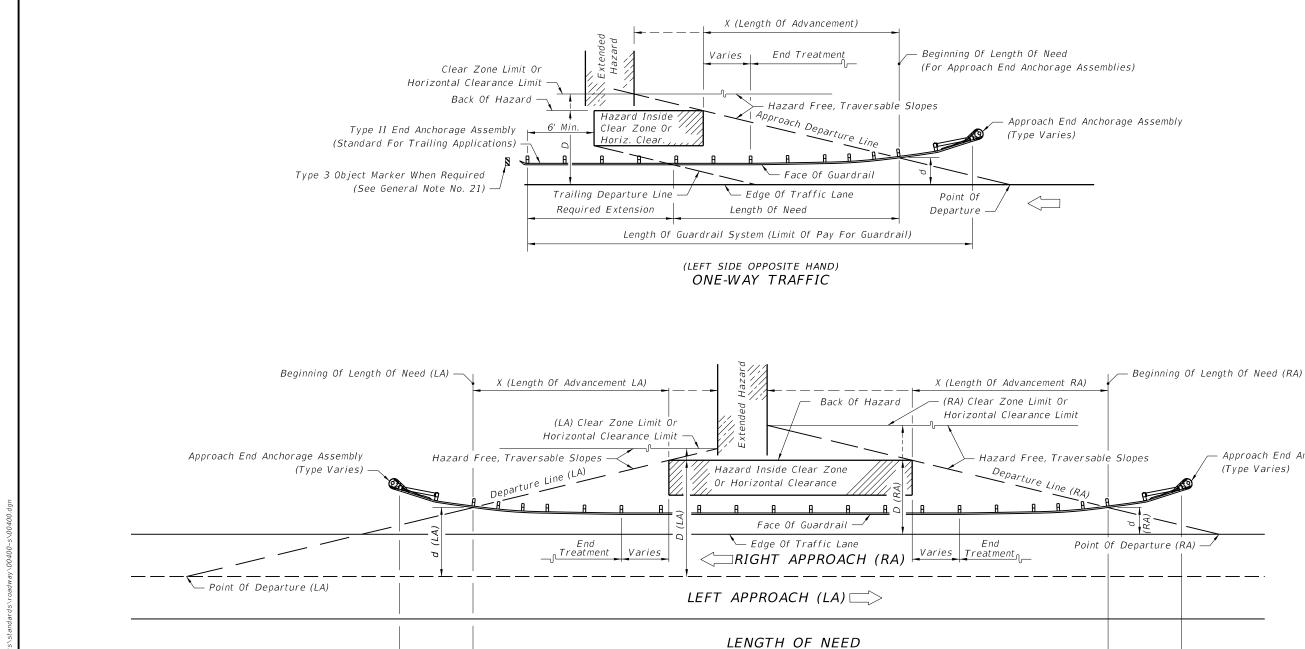
d = Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

For flared and parallel end anchorage assemblies the beginning length of need is to be set at the center of post #3. That is, the departure line must intersect the face of the rail at post #3.

For flared end anchorage assemblies the offset distance "d" will equal the normal guardrail offset measured from the face of the guardrail to the edge of the near approach travel lane plus 1'-2'' for 45 mph or less and $1'-9V_4''$ for greater than 45 mph.

LENGTH OF ADVANCEMENT - FIGURE 1

DESCRIPTION:



TWO-LANE TWO-WAY TRAFFIC

Length Of Guardrail System (Limit Of Pay For Guardrail)

For description of the dimensions D, d and X, see Length of Advancement - Figure 1. For additional shoulder guardrail information, see Details B and C.

LOCATING TERMINALS ON SHOULDER GUARDRAILS - FIGURE 2

DESCRIPTION: LAST REVISION 07/01/05

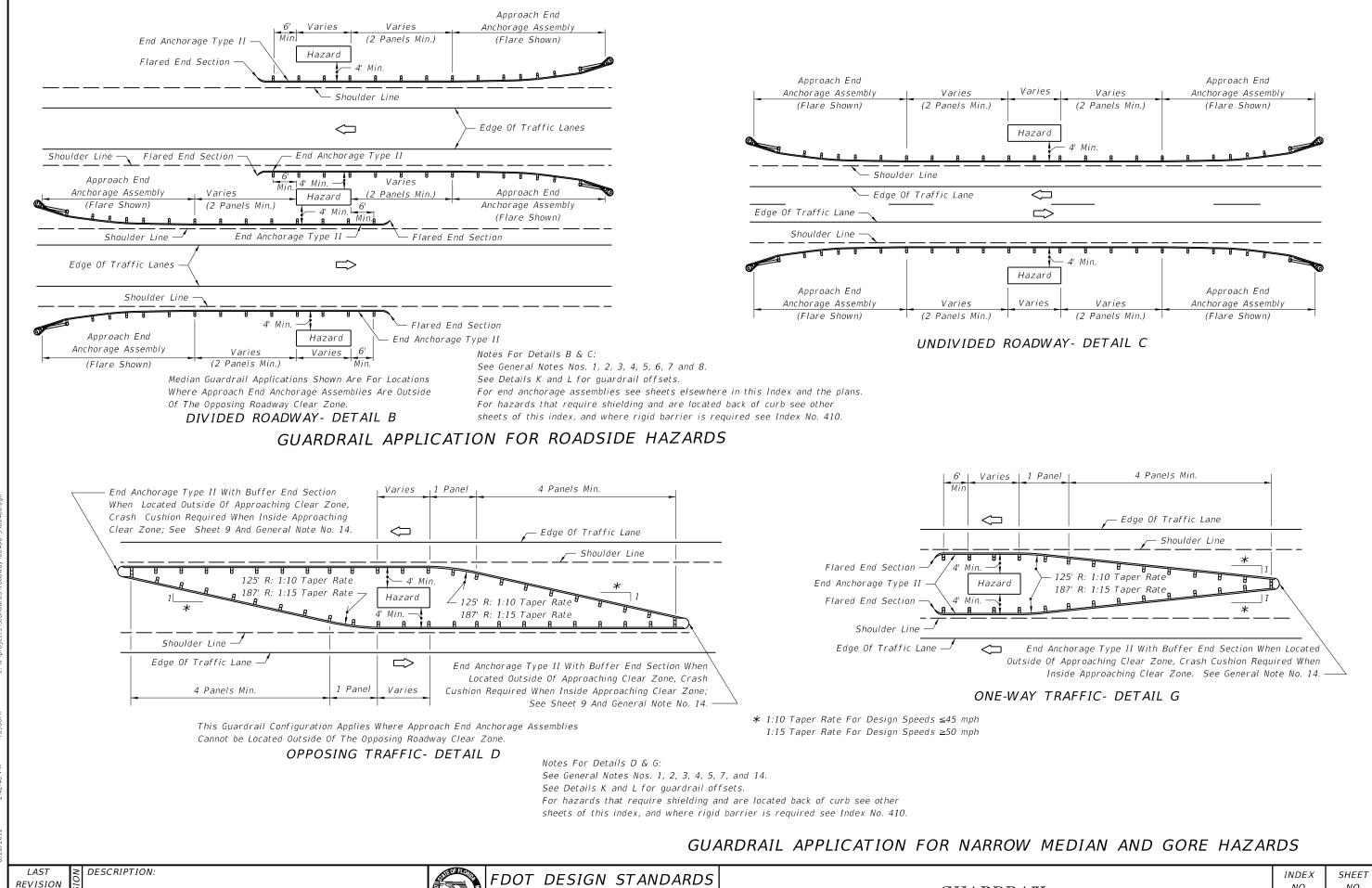
FDOT DESIGN STANDARDS 2013

GUARDRAIL

INDEX SHEET NO. NO. 400 4

Approach End Anchorage Assembly

(Type Varies)

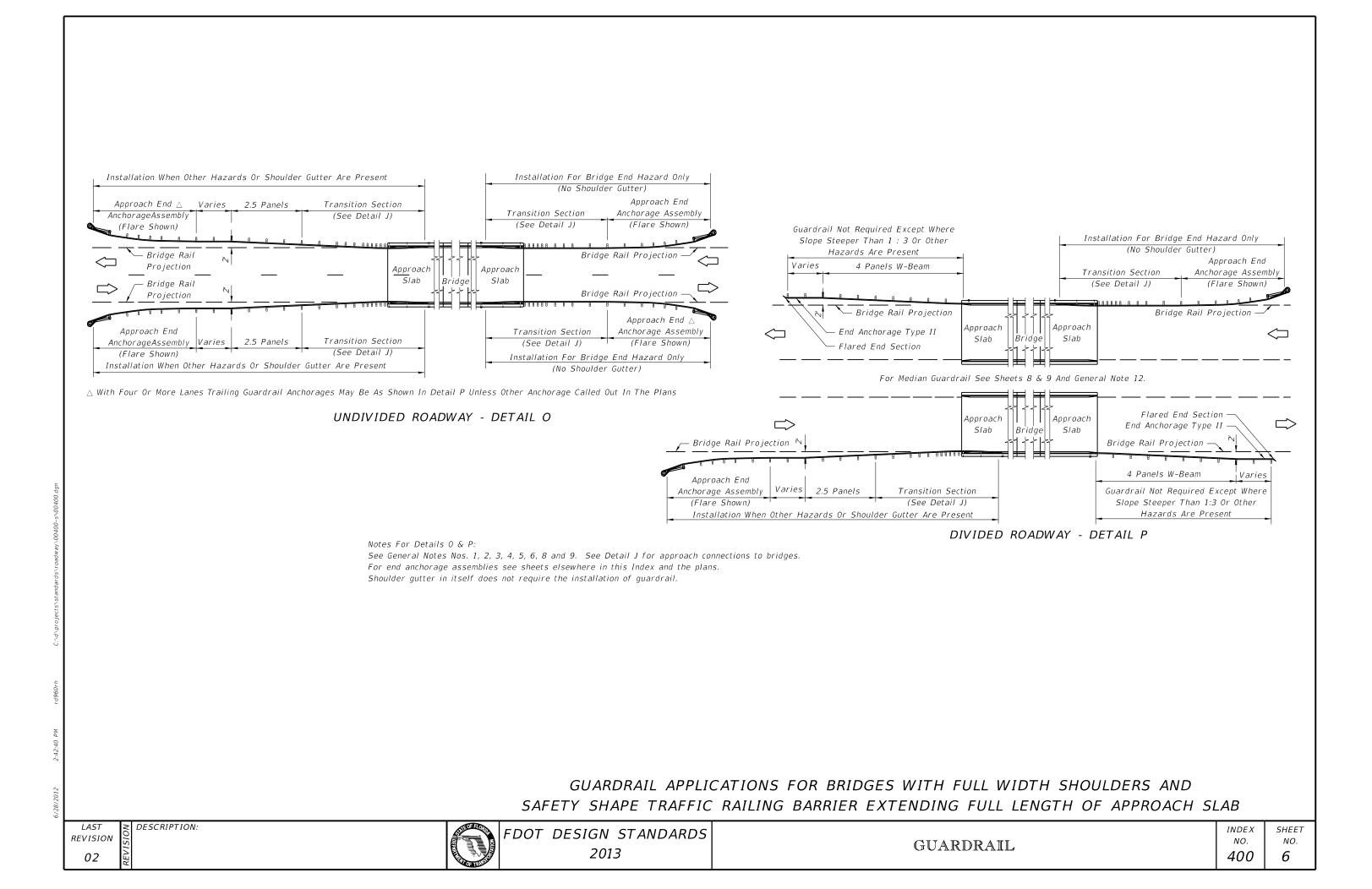


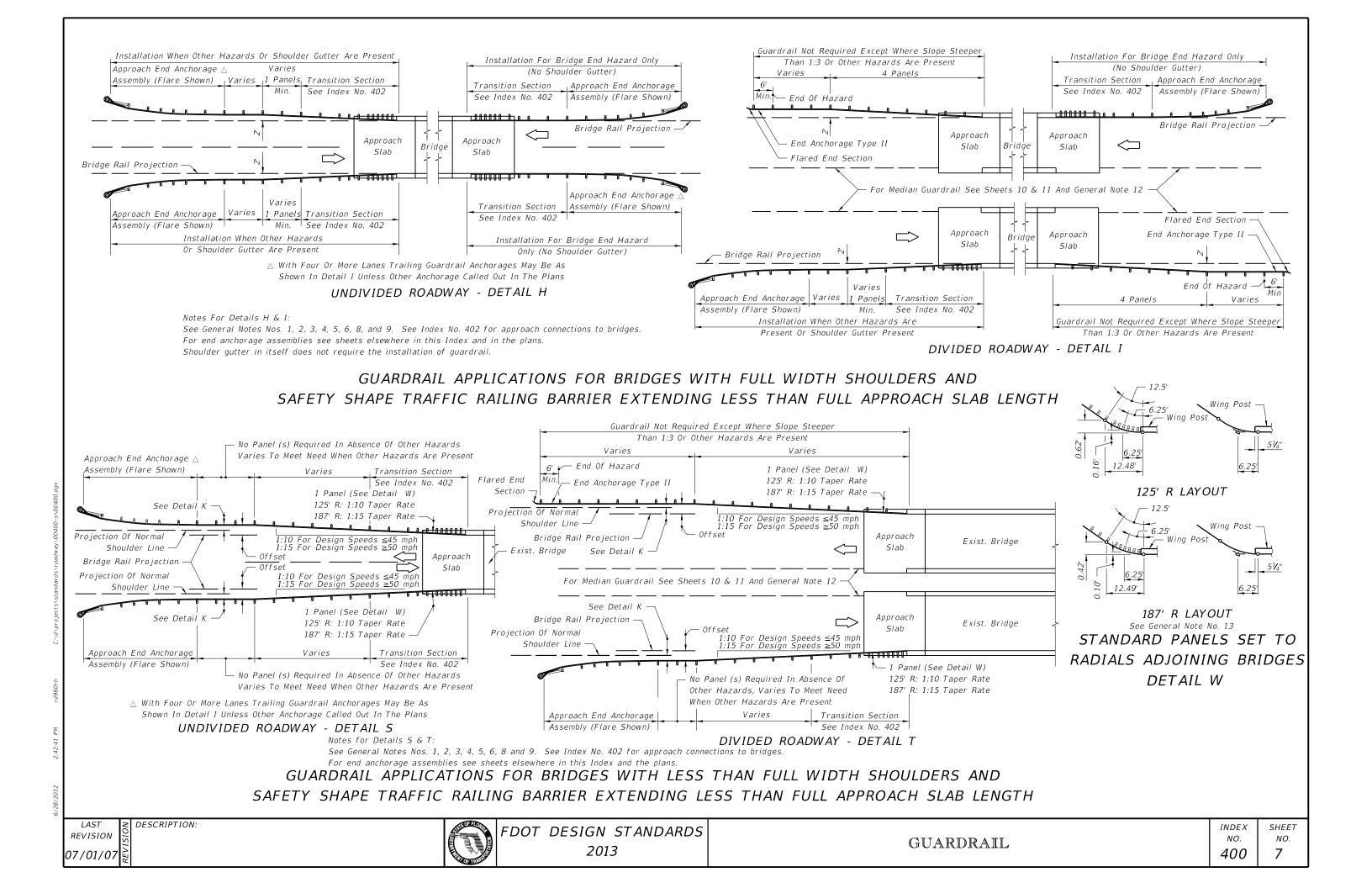
2013

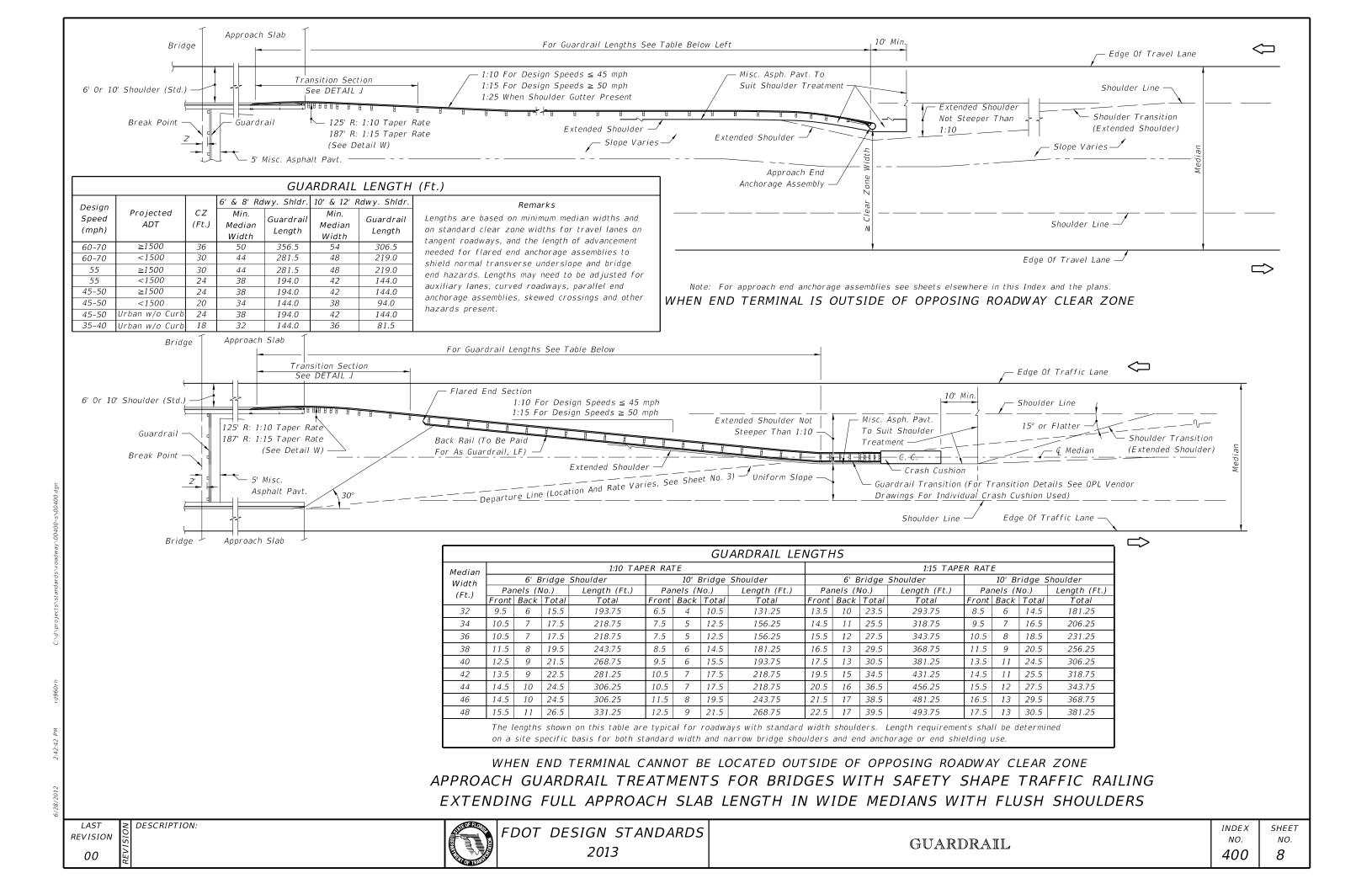
07/01/05

GUARDRAIL

NO. NO. NO. **400 5**



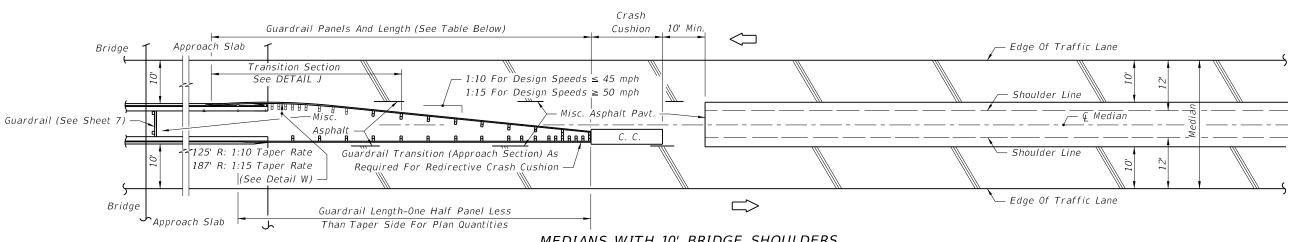




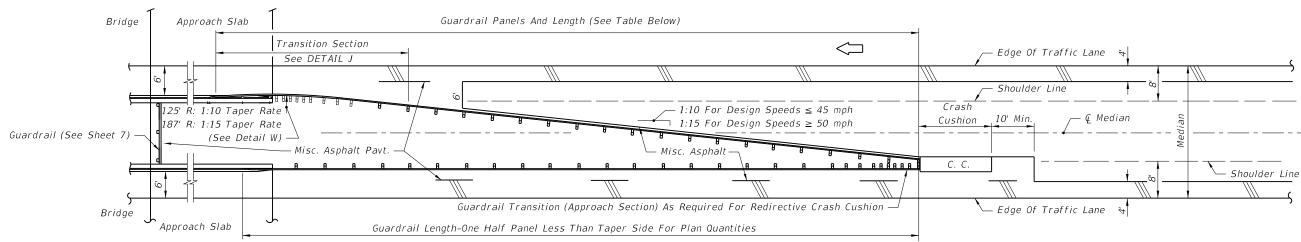






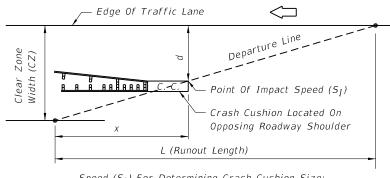


MEDIANS WITH 10' BRIDGE SHOULDERS



MEDIANS WITH 6' BRIDGE SHOULDERS □>

Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.



Speed (S_I) For Determining Crash Cushion Size: $S_I = \frac{x}{L} (Design Speed) = \left\lceil \frac{(CZ - d)}{CZ} \right\rceil Design Speed$

SIZING CRASH CUSHIONS LOCATED ON OPPOSING ROADWAY SHOULDERS

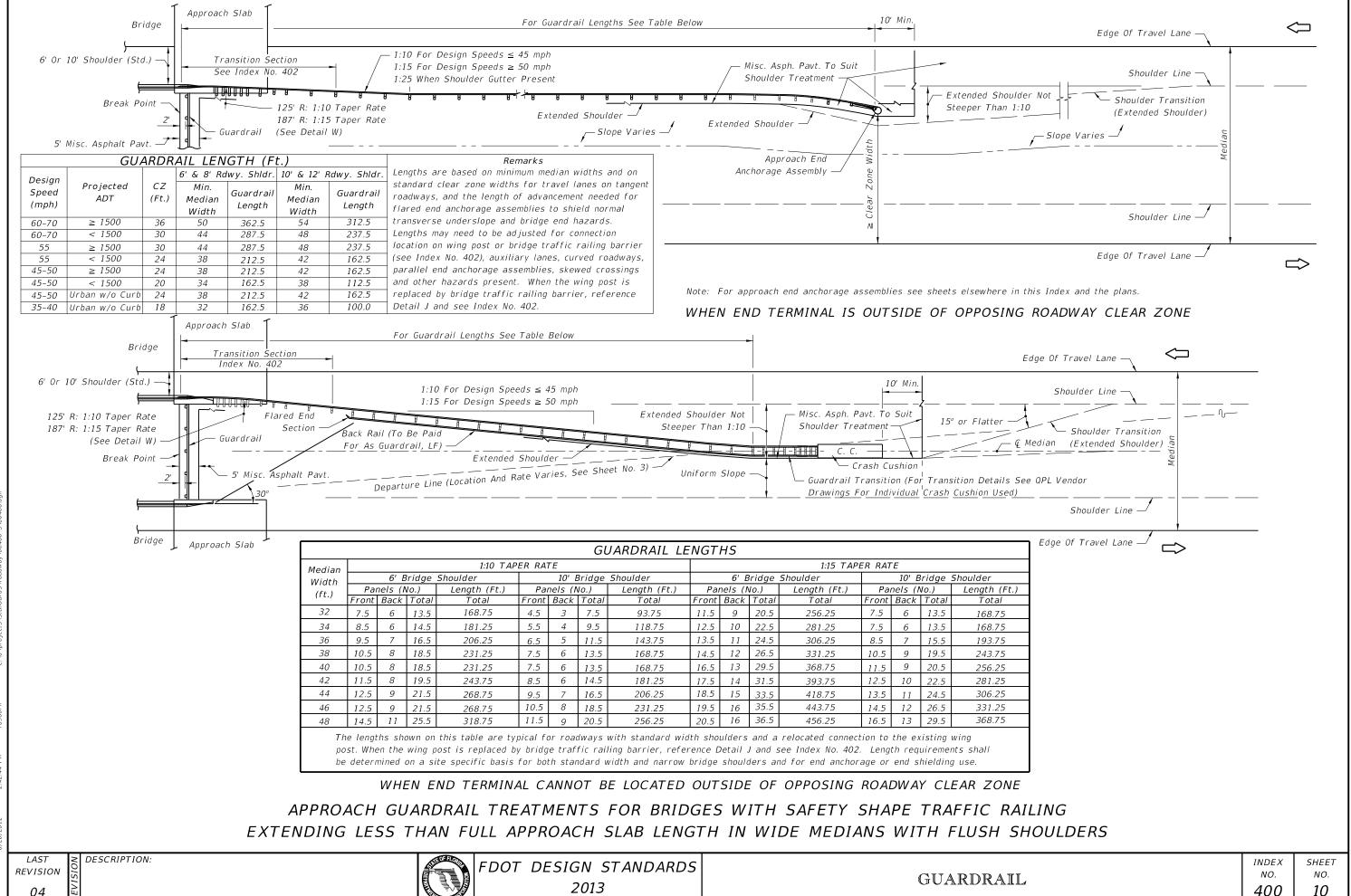
GUARDRAIL LENGTHS								
MEDIAN	N 6' BRIDGE SHOULDERS			10' BRIDGE SHOULDERS				
WIDTH	1:10 TAPER RATE 1:15 TAPER RATE		1:10 TAP	1:10 TAPER RATE		1:15 TAPER RATE		
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	14.5	181.25	20.5	256.25	7.5	93.75	10.5	131.25
28	12.5	156.25	18.5	231.25	6.5	81.25	8.5	106.25
26	11.5	143.75	15.5	193.75	5.5*	68.75	6.5	81.25
24	9.5	118.75	13.5	168.75	5.5*	68.75	5.5*	68.75

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds $(S_1's)$ along the runouts from the approach roadways; however, when calculated speeds $(S_1's)$ are less than 30 mph; crash cushions shall be no less in size than for 30 mph, see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width, see * below.

* Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

FDOT	DESIGN	STANDARDS 3
	201.	3





Edge Of Traffic Lane

L (Runout Length)

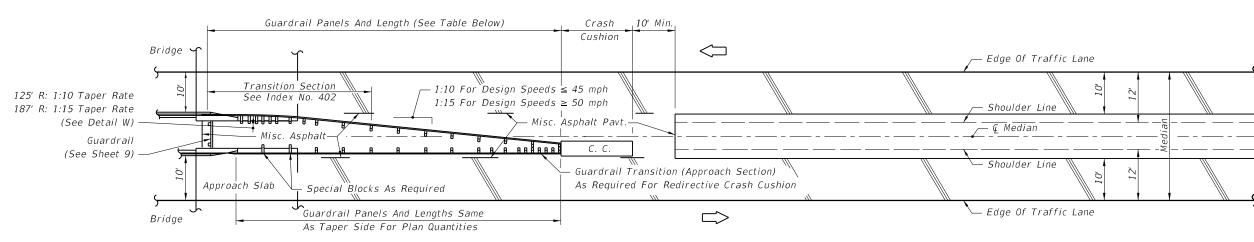
Speed (S_I) For Determining Crash Cushion Size:

 $S_I = \frac{x}{L} (Design Speed) = \left[\frac{(CZ - d)}{CZ}\right] Design Speed$

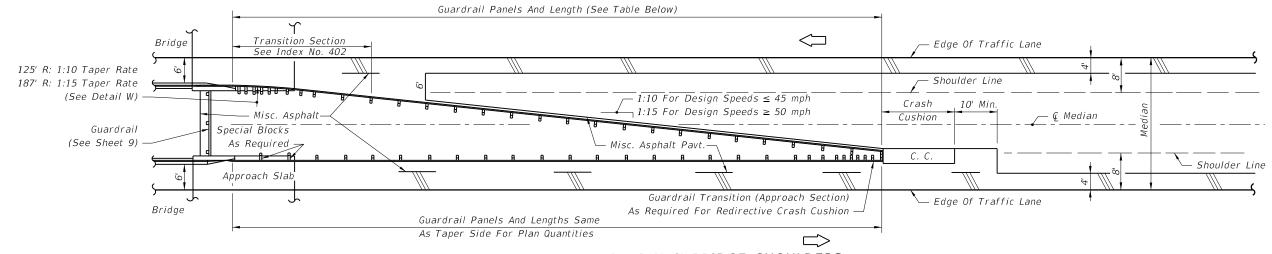
SIZING CRASH CUSHIONS LOCATED

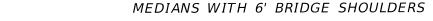
ON OPPOSING ROADWAY SHOULDERS

Point Of Impact Speed (S_I) Crash Cushion Located On Opposing Roadway Shoulder



MEDIANS WITH 10' BRIDGE SHOULDERS





Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

GUARDRAIL LENGTHS								
MEDIAN		6' BRIDGE	SHOULDERS		10' BRIDGE SHOULDERS			
WIDTH			ER RATE	1:10 TAPER RATE 1:15 TAPER R.		ER RATE		
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	12.5	156.25	18.5	231.25	6.5	81.25	9.5	118.75
28	11.5	143.75	16.5	206.25	5.5	68.75	7.5	93.75
26	9.5	118.75	14.5	181.25	5.5*	68.75	5.5*	68.75
24	8.5	106.25	11.5	143.75	5.5*	68.75	5.5*	68.75

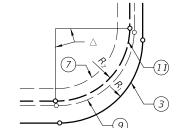
The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds $(S_1's)$ along the runouts from the approach roadways; however, when calculated speeds $(S_1's)$ are less than 30 mph crash cushions shall be no less in size than for 30 mph; see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see * below.

*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

LAST

DESCRIPTION:



No. 3 post for Radii > 25' and < 50'.

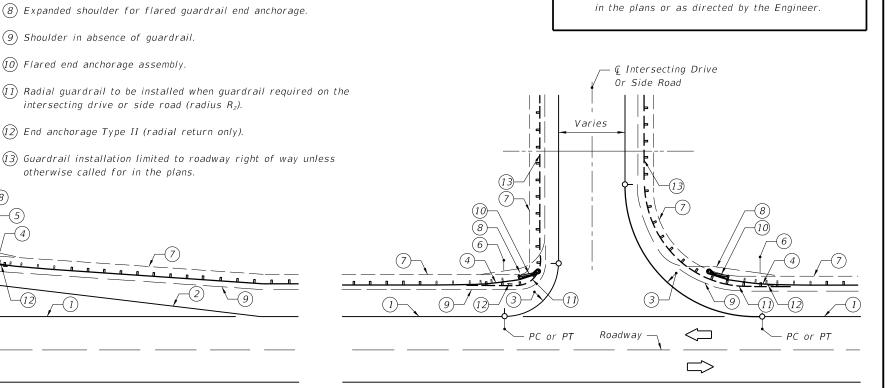
(6) Post for locating flare, proximate to PC or PT:

otherwise called for in the plans.

Between No. 4 and No. 5 posts for Radii 50' or greater.

RADIAL GUARDRAIL

	No. 3 post for Radii 25' or less. Between No. 4 and No. 5 posts for Radii greater than 25'.
	7) Expanded shoulder for guardrail.
_— ← Intersecting Drive	(8) Expanded shoulder for flared guardrail end anchorage.
√ Or Side Road	Shoulder in absence of guardrail.
	(10) Flared end anchorage assembly.
	(1) Radial guardrail to be installed when guardrail required on the intersecting drive or side road (radius R2).
	(2) End anchorage Type II (radial return only).
∴ − − − − − − − − − − − − − − − − − − −	



RADIAL GUARDRAIL Normal Turnouts

85°56′

85°56′

85°56′

85°56′

85°56′

89°31′

89°31′

89°31′

Note: Only 25' and 40' radius panels are to be used for

return guardrail on normal turnouts. On skewed turnouts the number of panels used and their arrangement with straight panels will be as shown

25'

25'

25'

25'

40'

40'

40'

Taper

Panels

Required

 R_2

40'

40'

20' 25'

25' 25'

30' 25'

35' 25'

40'

Simple Curve

85°56′

85°56′

85°56′

85°56′

85°56′

89°31′

89°31'

89°31'

Panels

Required

TAPER TURNOUTS

 \Box

Roadway

Note: The guardrail application shown on this sheet are for highways with flush shoulders and no restraints for constructing flared end anchorages and minimum lengths of guardrail. For highways with flush shoulders and restraints to constructing flared anchorages, see General Note No. 6.

Where openings in guardrail are required in close proximity to bridge traffic rails or ends of concrete barrier walls, and minimum length guardrail with flared end anchorages can not be applied, either controlled release returns or energy absorbing terminals are to be applied.

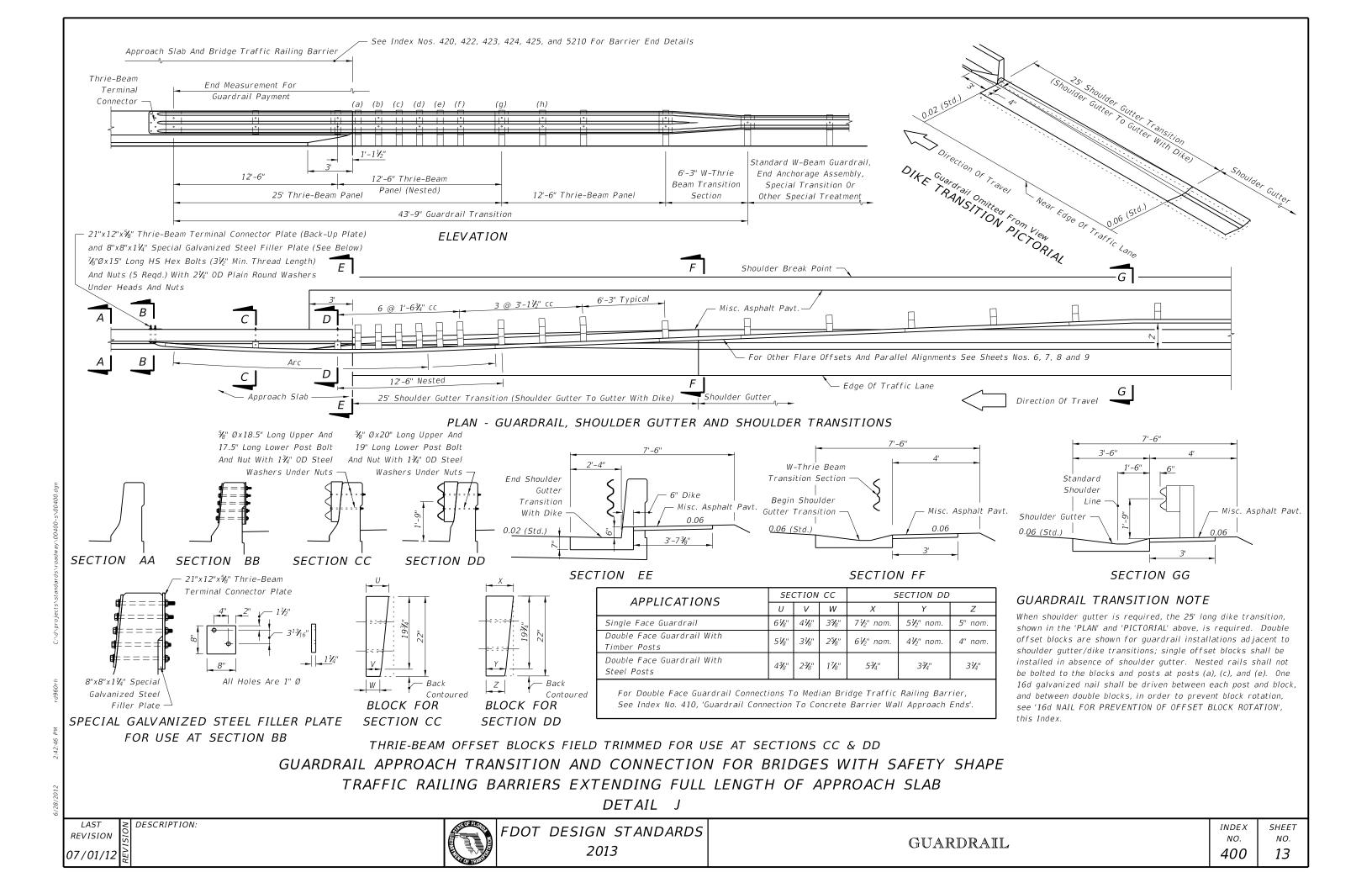
SIMPLE CURVE TURNOUTS

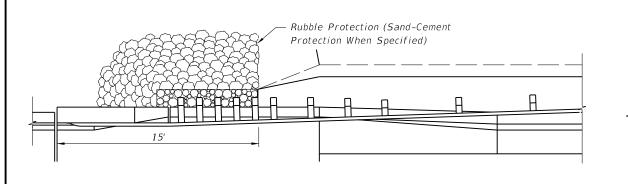
GUARDRAIL APPLICATIONS FOR INTERSECTING DRIVES AND SIDE ROADS ON RURAL FACILITIES

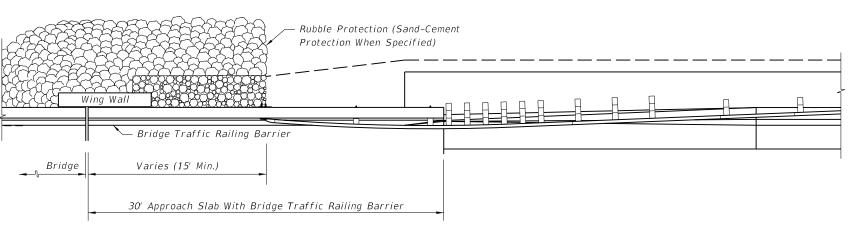
LAST REVISION 04

DESCRIPTION:



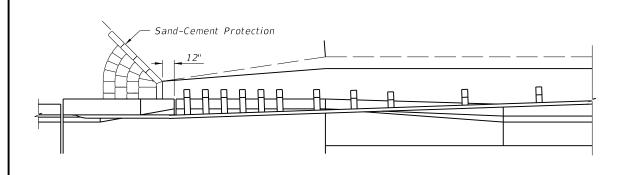


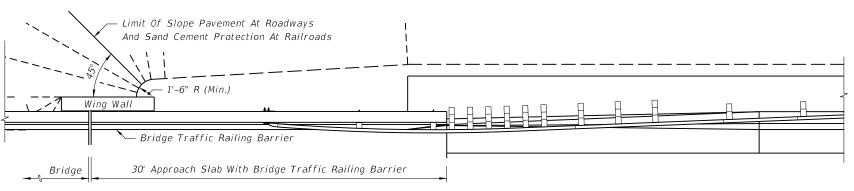




BRIDGES OVER STREAMS

BRIDGES OVER STREAMS





BRIDGES OVER RAILROADS

BRIDGES OVER ROADWAYS OR RAILROADS

For Additional Information See Index No. 402

For Additional Guardrail Information See Sheet 13

SKETCHES - BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING LESS THAN FULL APPROACH SLAB LENGTH SKETCHES - BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING FULL APPROACH SLAB LENGTH

SKETCH NOTES

- 1. These sketches are for showing shoulder interface between roadways and bridges where crossings are normal to other roadways, railroads and streams. For site specific applications and details see the plans and the FDOT Structures Design Office "Detailing Manual" and "Design Guidelines".
- 2. Shoulder treatments shown in these sketches are for locations with shoulder gutter; shoulder hinge location will vary for facilities without shoulder gutter.

SHOULDER INTERFACE BETWEEN ROADWAYS AND BRIDGES

LAST REVISION 04

DESCRIPTION:

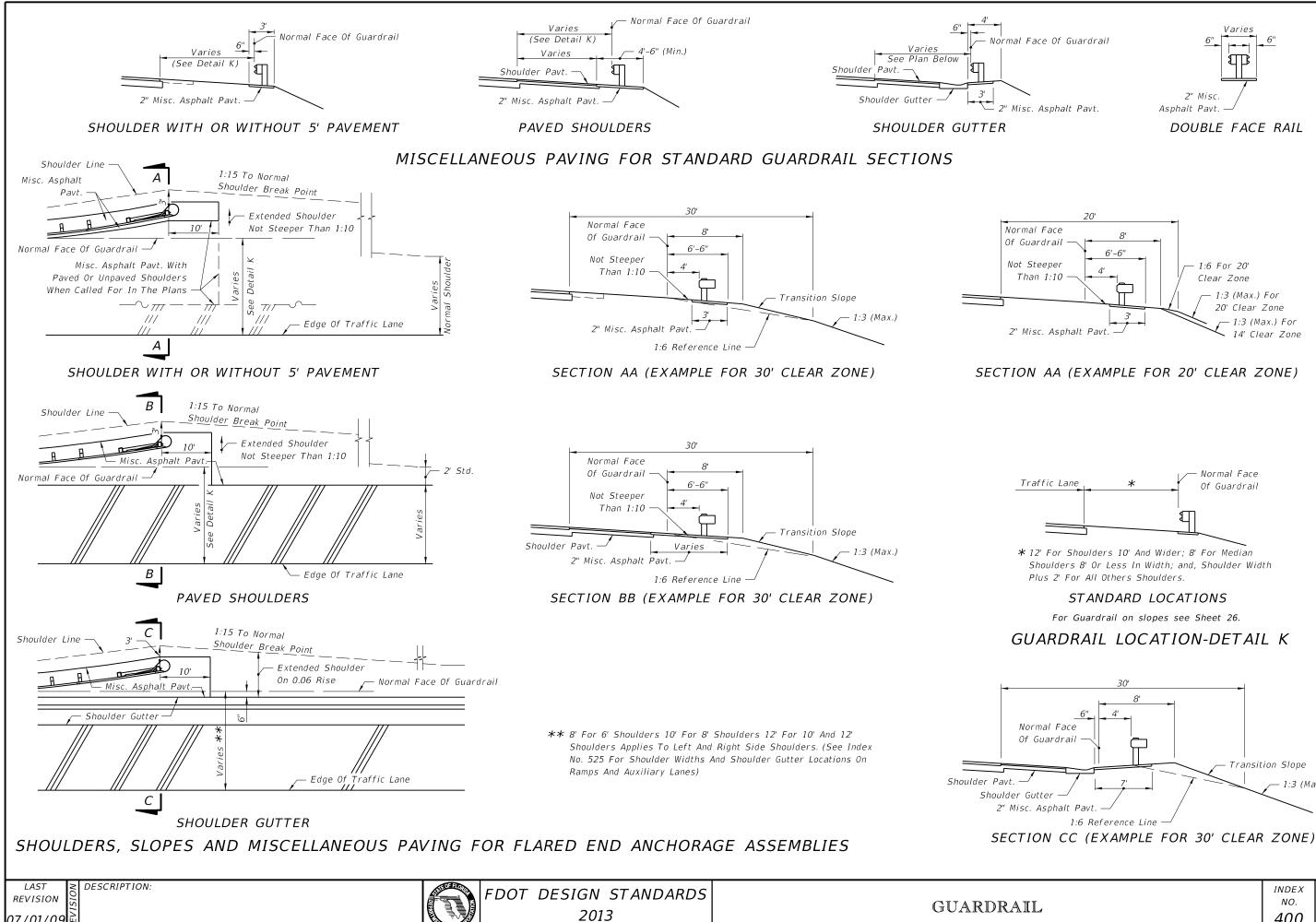


DOT DESIGN STANDARDS 2013

GUARDRAIL

INDEX NO. 400

SHEET NO. 14



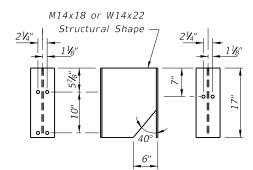
07/01/09

SHEET INDEXNO. NO. 400 15

1:3 (Max.)

14' Clear Zone

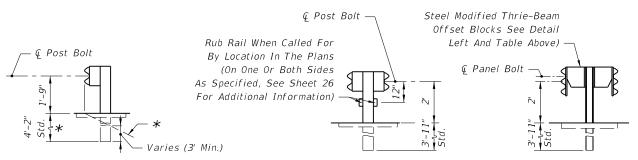
STEEL POST



POST TRAFFIC SIDE VIEW FACE FACE

All Holes Are 13/16"Ø

STEEL MODIFIED THRIE-BEAM OFFSET BLOCK



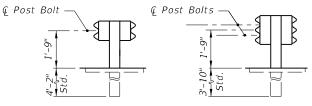
16d Galvanized Nail

Timber Or Recycled

Plastic Block

W-BEAM W-BEAM WITH RUB RAIL

MODIFIED THRIE-BEAM



THRIE-BEAM

* Front Slope When Right Of Way, Environmental Or Other Restrictions Prohibit Normal Shoulder Extension

SINGLE FACED GUARDRAIL

DESCRIPTION:

For Narrow Medians With No Median Swale. See Sheet 26 For Median With Swale

> THRIE-BEAM W-BEAM

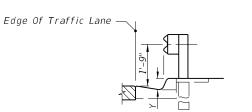
DOUBLE FACED GUARDRAIL

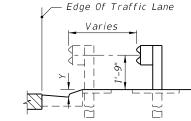
MOUNTING HEIGHTS ON SHOULDERS AND IN MEDIANS

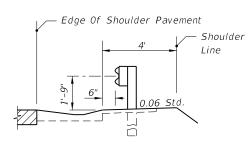
PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS POSTS OFFSET BLOCKS Post bolt hole in timber and plastic blocks to be centered $\pm \frac{1}{4}$ "). Timber 6"x8"x14" (Nominal) For W-Beam And 6"x8"x22" All timber offset blocks shall be dressed on all four sides (S4S). (Nominal) For Thrie-Beam Timber One 16d galvanized nail per block is to be used to prevent rotation Recycled Plastic (See Notes) of block (see detail left). Timber 6"x8"x14" (Nominal) For W-Beam And 6"x8"x22" Same as above for timber and plastic blocks except that form fit Steel (Nominal) For Thrie-Beam plastic block holes align with holes in steel posts and do not W6x8.5, W6x9 Or 6" C Recycled Plastic (See Notes) $\frac{1}{8}$ "Øx $1\frac{1}{2}$ " long hex head bolts with full length thread and nuts W14x22x17" (M14x18x17") Steel (2 Reqd.) and ⅓" plain round washers (4 Reqd.) for mounting W6x8.5, W6x9 Or 6" C (Steel Modified Thrie-Beam) steel block to post. Bolts are to be installed in opposite holes, top and bottom.

Notes: 1. Timber and recycled plastic offset blocks of identical size and shape can be intermixed within a run of rail.

2. Recycled plastic offset blocks shall meet the passing evaluation criteria for Test Level 3 crash tests. The blocks shall be tested as a component in a semi-rigid guardrail test article under full scale crash test conditions. The blocks shall be in conformance with Sections 536 and 972 of the Specifications and be included on the Qualified Products List. W-Beam blocks shall be 14" in height and thrie-beam blocks shall be 22" in height. The blocks shall be capable of providing a 7½" (Min.) offset. For additional information on plastic offset block installations refer to Sheet 1, GENERAL NOTE 16.







Y = 6" Or Greater

Y = Less Than 6"

Shoulder Gutter

For location of guardrail with offset behind curb and gutter refer to the Plans Preparation Manual, Volume 1, Section 4.3.5.

LOCATION AT CURB & GUTTER SECTIONS-DETAIL L

LAST REVISION 07/01/09

© Post Bolts

DOT DESIGN STANDARDS 2013

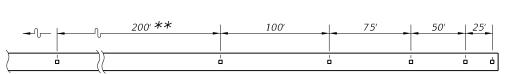
GUARDRAIL

INDEX NO. 400 SHEET NO. 16

NOTES



DESCRIPTION:



Note: Adjustment in spacing may be required to fit exact guardrail lengths as directed by the Engineer. For minimum installations (length 62.5') provide one reflector at each end and one at the approximate center.

REFLECTOR ELEMENT SPACING

**For curves greater than 2° the spacing shall be reduced to 100' increments through the curve.

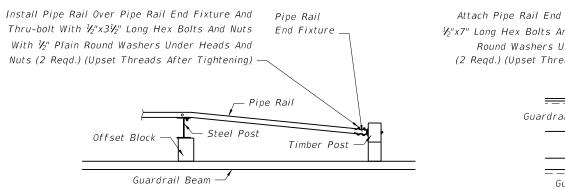
Steel Post Wood Post

PICTORIAL VIEW REFLECTOR MOUNTING

REFLECTORS-DETAIL M

REFLECTOR NOTES

- 1. Reflectors shall conform to Section 993 of the Standard Specifications.
- 2. Reflector color (white or yellow) shall conform to the color of the near lane edgeline.
- 3. Reflectors installed on median guardrail shall have retro-reflective sheeting on both sides of the reflector.
- 4. The cost for reflectors shall be included in the contract unit price for



PLAN

Attach Pipe Rail End Fixture To Post With This Post Shall Be Timber Only ½"x7" Long Hex Bolts And Nuts With ½" Plain Round Washers Under Heads And Nuts (2 Reqd.) (Upset Threads After Tightening) -Steel Post -Guardrail Beam Top Of Curk Pipe Rail End Fixture Gutter —

ELEVATION

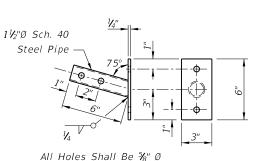
⅓"Ø Bracket And Pipe Holes With ½"x3½" NPS 2 Sch. 40 Galv. Pipe Rail Per Long Hex Bolt And Nut With 1/2" Plain Round ASTM F1083 Washer (Upset Threads After Tightening) ¾"Ø Bracket Hole With ¾"x2" Long Hex Bolt And Nut With ⅓" Plain Round Washers (Upset Threads After Tightening) Steel Guardrail Post Steel Guardrail Post $2\frac{1}{2}$ "x2"x\frac{1}{4}"x4" Long Angle 11/8" Offset From € Bracket (Galvanized) Of Guardrail Post VIEW A VIEW B

PIPE RAIL MOUNTING

1. Pipe Rail is required on steel guardrail posts when the front of sidewalks or shared use paths are located 4' or less from behind the back of the post. Pipe rail shall not extend beyond the last post of the approach end anchorage assembly. Begin and end the Pipe Rail in accordance with the PIPE RAIL END FIXTURE detail.

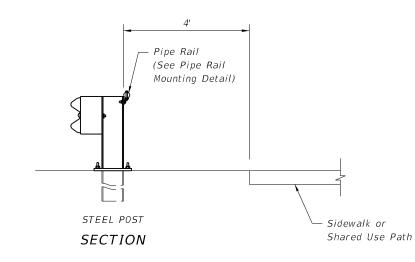
Refer to Sheet 1, GENERAL NOTE 6 for guardrail end treatment requirements.

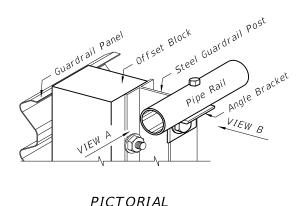
- 2. When guardrail with timber posts are located with the back of post 4' or less from the near front of sidewalks or shared use paths, the bolt ends will require one of the following treatments:
- a. Trim back flush with the face of nut and metalize or
- b. Use post bolts 15" in length with washers and nuts counter sunk into sinks 1" to $1\frac{1}{2}$ " deep or
- c. Use post bolts 15" in length with sleeve nuts and washers.



Galvanize After Drilling And Welding

PIPE RAIL END FIXTURE



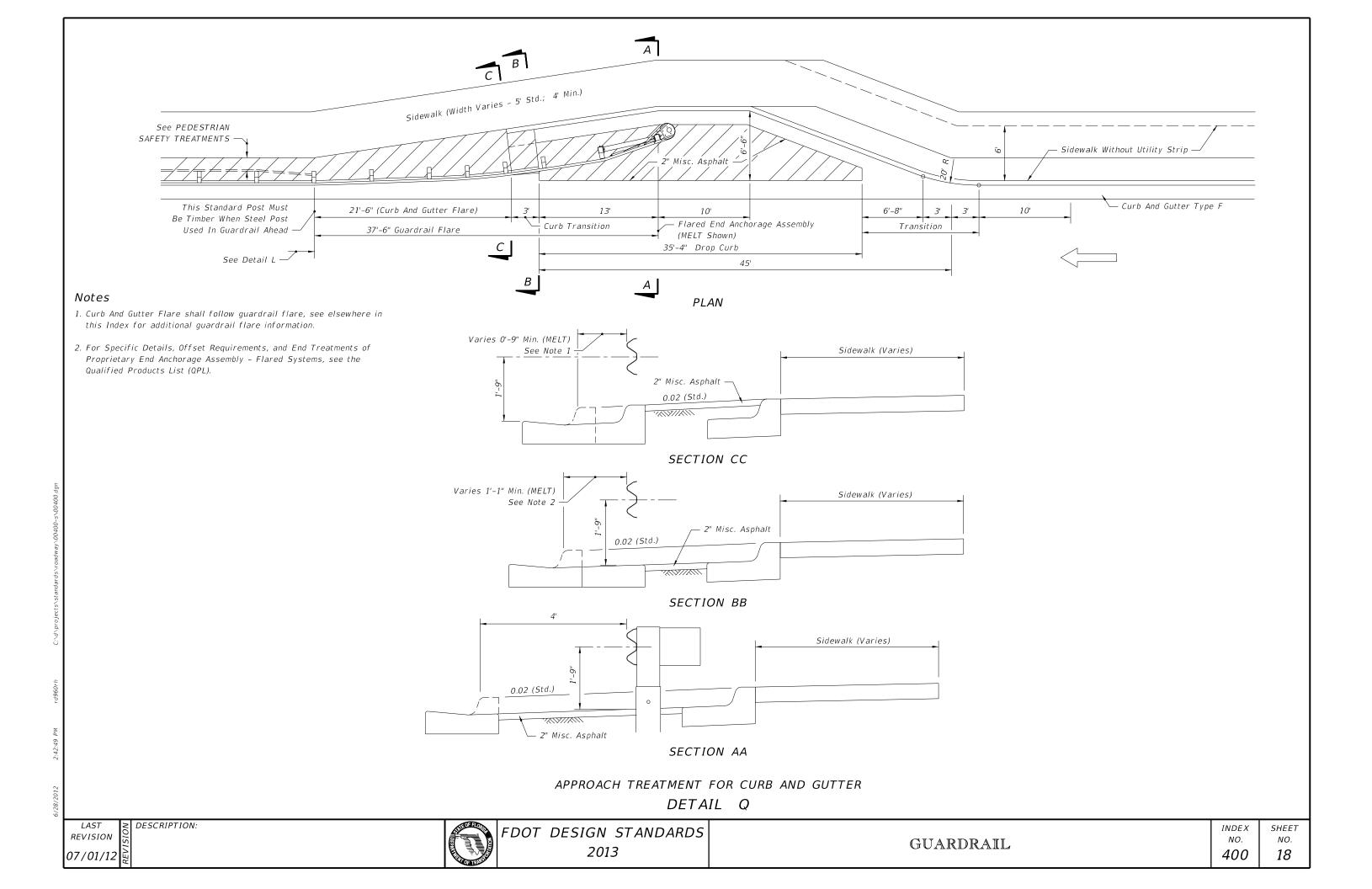


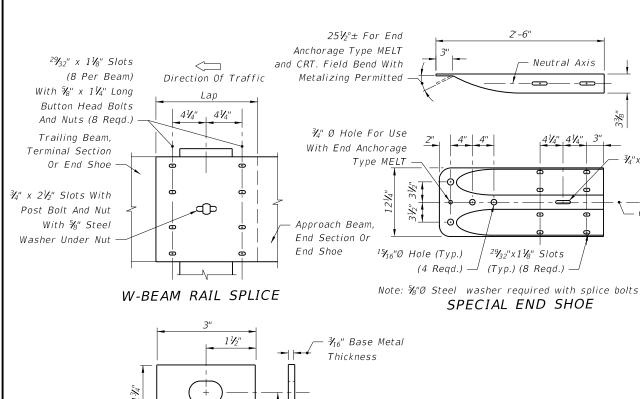
FOR LOCATIONS USED BY PEDESTRIANS OR BICYCLISTS

PEDESTRIAN SAFETY TREATMENTS

DOT DESIGN STANDARDS 2013

SHEET *INDEX* NO. NO. 17 400





1"Øx⅓16" Deep Recess (Both Sides) 5/8" MODIFIED HEAVY HEX NUT (RECESSED NUT)

¾"x2½" Slot —

29/32"x11/8" Slots

(Typ.) (8 Regd.)

FLARED END SECTION

3/4"x21/3" Slot

3/4"x21/2" Slot -

ROUNDED END SECTION

Note: For application information see individual end anchorage assembly details.

(RECTANGULAR PLATE WASHER) BEAM WASHER

11/16"x1" Slot

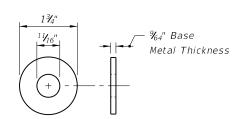
For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary

to accomplish alignment or where the posts bolt head shows tendency

to pull through the rail slot. Washers installed on guardrail, between

end anchorages, prior to July 1, 1990 may remain in place until the

guardrail is relocated or until repairs require removal and



Note:

reinstallment of a post bolt.

The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by 🐉 Ø hex bolts and nuts and under hex nut for connecting rub rail to wood and steel posts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

5/8" STEEL WASHER

THREAD LENGTH **APPLICATION** (In.) 'Min.) (In.) 11/4" Full Length | Rail Splice Bolt Single Or Double Faced Guardrail Timber Or Recycled Plastic Offset 10" Block(s) On Steel Post Post Bolt -As An Option, A Single 25"★ Long Post Bolt May Be Used 18" Post Bolt - Single Faced Guardrail Timber Posts Post Bolt - Double Faced Guardrail Timber Posts 25" Double Faced Guardrail Steel Posts

Special bolts having lengths of 10" or greater shall have a thread length of not less than 4".

For applications where special bolts having lengths greater than 25" are required, the Contractor may use a 5/8"Ø threaded rod (field cut to length). A hex nut and beam washer shall be used at the guardrail face with no more than $\frac{3}{4}$ " of the threaded rod projecting beyond the top of the nut. The projecting thread on both ends shall be distorted to secure the nuts, and both ends of the threaded rod metalized with organic zinc-rich coating.

stUse of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced guardrail that results in the bolt projecting more than $\frac{3}{4}$ " beyond the face of the nut after pull-up shall be trimmed to \mathcal{Y}_4 " reveal and metalized with organic zinc-rich coating.

%" OVAL SHOULDER BUTTON HEAD BOLT

W-BEAM BACK-UP PLATE

2'-4"

OFFSETS (Ft.)					
Measured From Face Of Guardrail To					
Front Of Above Ground Rigid Hazard					
POST	SINGL	E BEAM	NESTED BEAMS		
SPACING (Ft.)	W-Beam	Thrie-Beam	W-Beam	Thrie-Beam	
6'-3"	4'-0" 3'-4"		N/A	N/A	
3'-1½"	3'-0"	2'-8"	2'-8"	2'-4"	

N/A

Contour To Fit

Over Beam

29/32" x 11/8" Slots

(Typ.) (4 Reqd.)

Ф

81/2"

·Varies

Note:

1'-6¾''

The values shown should be utilized unless changes are supported by empirical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5.4 of the AASHTO Roadside Design Guide are cautioned to proceed only if background in the table development is

N/A

MINIMUM OFFSET FOR SINGLE FACED GUARDRAIL (Ft.)

HS Hex bolts for THRIE-BEAM TERMINAL CONNECTORS shall conform to the requirements of ASTM A449 (Type 1) with heavy hex nuts and washers. All other hex bolts shall conform to the requirements of ASTM A563. Bolts, nuts and washers shall be hot dip galvanized. Heavy hex nut may be used in lieu of hex nuts and hex nuts used for jam nuts.

²%₃₂"x1%" Slot 3/4"x21/2" Slot 10°(± 1°) Sheet Tolerance 33/16" (-0,+ ½₁₆") 31/4"

W-BEAM

1'-3" R Standard

Type MELT)

(10½" R When Used

For End Anchorage

7½"

BUFFER END SECTION

Contour To Fit

Over Beam

²9/₃₂"x11/8" Slots

(Typ.) (4 Reqd.)

81/2"

HEX BOLTS AND NUTS

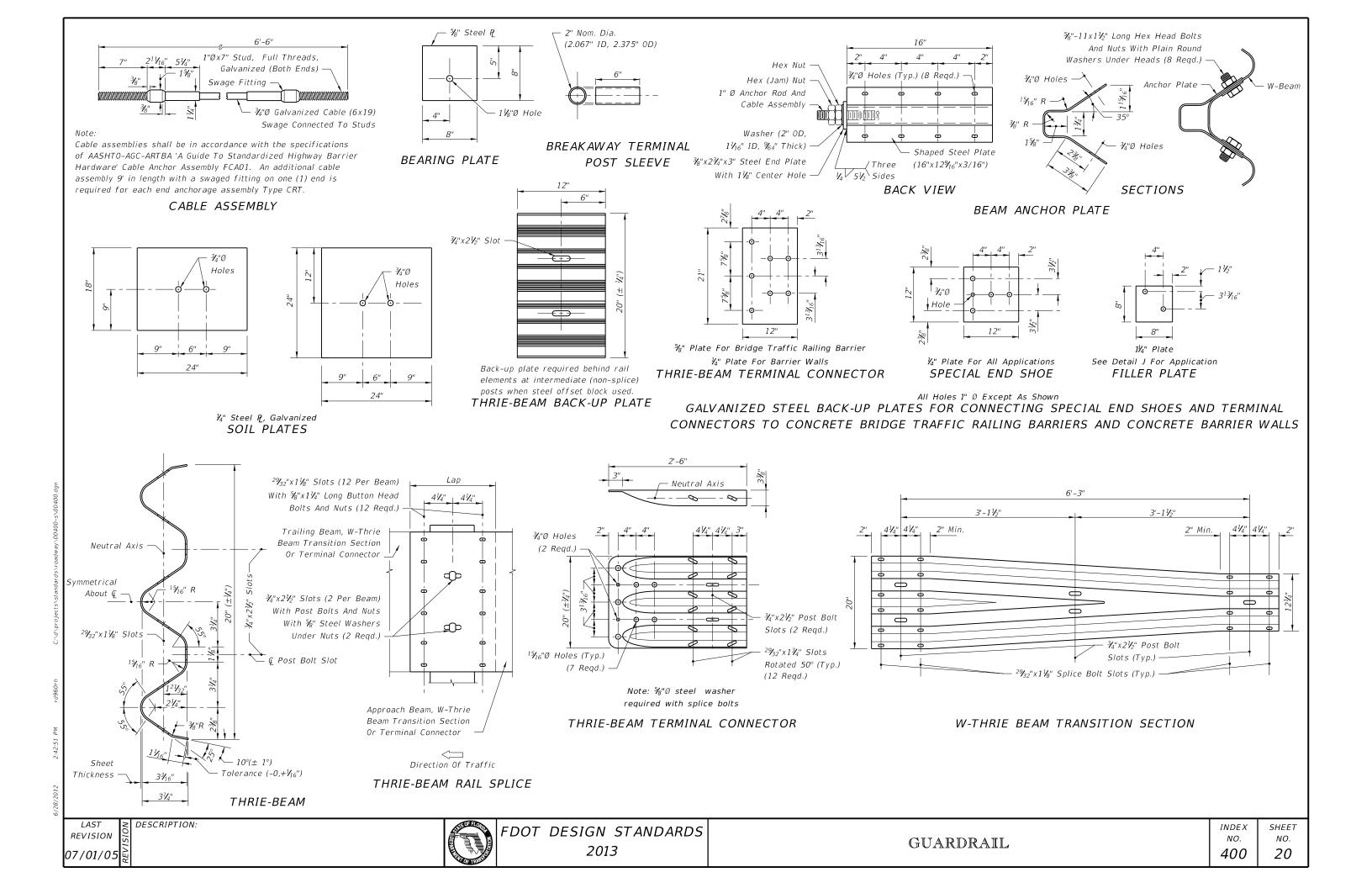
LAST DESCRIPTION: REVISION 07/01/07



DOT DESIGN STANDARDS 2013

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GUARDRAIL

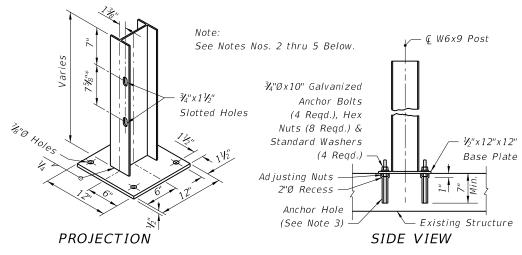


FOR REPLACEMENT OF EXISTING W8x18 GUARDRAIL POSTS ON APPROACH SLABS AND BRIDGES

* Additional slotted hole required when mounting thrie-beam guardrail

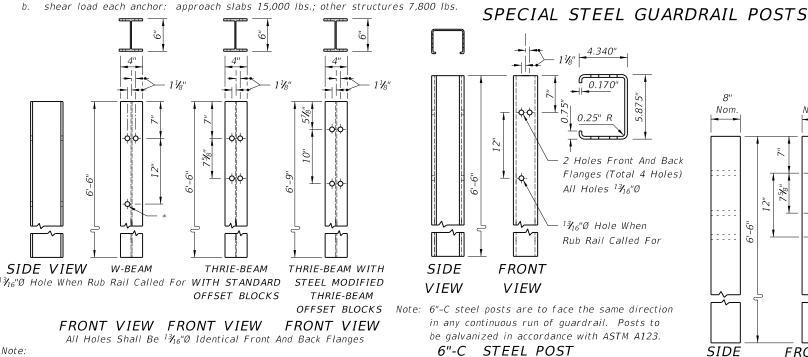
NOTES: (SPECIAL STEEL POST)

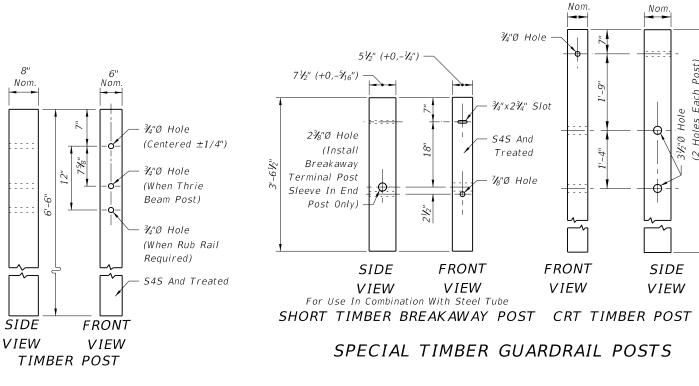
- 1. See Index No. 402 for special steel posts required for construction and repair of quardrail transitions to bridge traffic railing barrier retrofits on existing bridges. See Structures Index Nos. 470 through 476 for steel posts required to construct traffic railing barrier retrofits on existing bridges.
- 2. Either anchor bolts, concrete wedge anchors or approved Adhesive-Bonded Anchors for Structural Applications may be used. Anchor bolts, wedge anchors and adhesive anchors shall have a minimum tensile strength of 60,000 psi and galvanized in accordance with ASTM A153 (stainless steel components may be substituted but components plated in accordance with ASTM B-633 are not acceptable). Adhesive anchor rods shall be equal in diameter to that detailed for anchor bolts. Wedge anchors are to be installed in accordance with the manufacturer's recommendations, assuming 3,000 psi compressive strength for concrete. Wedge anchors shall also meet the following requirements:
- a. tensile load each anchor: approach slabs 14,000 lbs.; other structures 8,000 lbs.
- b. shear load each anchor: approach slabs 15,000 lbs.; other structures 7,800 lbs.



FOR CONSTRUCTION OF GUARDRAIL WHERE CULVERT, PIER FOOTING OR OTHER STRUCTURE PRECLUDES DRIVEN POST INSTALLATION

- 3. Posts are to be plumbed by adjusting nuts or mortar seating. Posts installed using anchor bolts and adhesive anchors are to be set with adjusting nuts as detailed, unless the Engineer approves the use of mortar seating in lieu of adjusting nuts. Posts installed using wedge anchors are to be set with mortar seating. Base plates shall be grouted with neat finish.
- 4. Adhesive-Bonded Anchors for Structural Applications shall comply with Section 937 and be installed in accordance with Section 416. Drilled hole diameter shall be in accordance with the manufacturer's instructions.
- 5. Anchor holes and recesses shall be drilled; wedge anchor holes are to be drilled in accordance with the manufacturer's specifications. Encountered reinforcing steel shall be drilled through. Holes shall be thoroughly cleaned when setting bolts and anchors and dry when setting wedge anchors.
- 6. Steel post and base units shall be galvanized in accordance with ASTM A123. Any damaged galvanized areas are to be metalized in accordance with Section 562 of the Standard Specifications.
- 7. Special steel posts are not to be substituted for any post in a quardrail approach end treatment system.





3/4"0 Holes

TS 8"x6"x¾₁₆"

Galvanized

Open End

seal welded between web and flange before galvanizing. Posts to be galvanized in accordance with ASTM A123. W6x8.5 OR W6x9 STEEL POST STANDARD TIMBER AND STEEL GUARDRAIL POSTS

W6x8.5 or W6x9 steel posts may be either rolled or welded structural shapes conforming to or exceeding the design properties of

ASTM A6/A6M. Welding shall be in accordance with the requirements of ASTM A769/A769M. Posts shall be cut to length and the ends

LAST REVISION 07/01/09

DESCRIPTION:

FDOT DESIGN STANDARDS 2013

GUARDRAIL

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hen Us BEST

W Of

(6'-6" Part

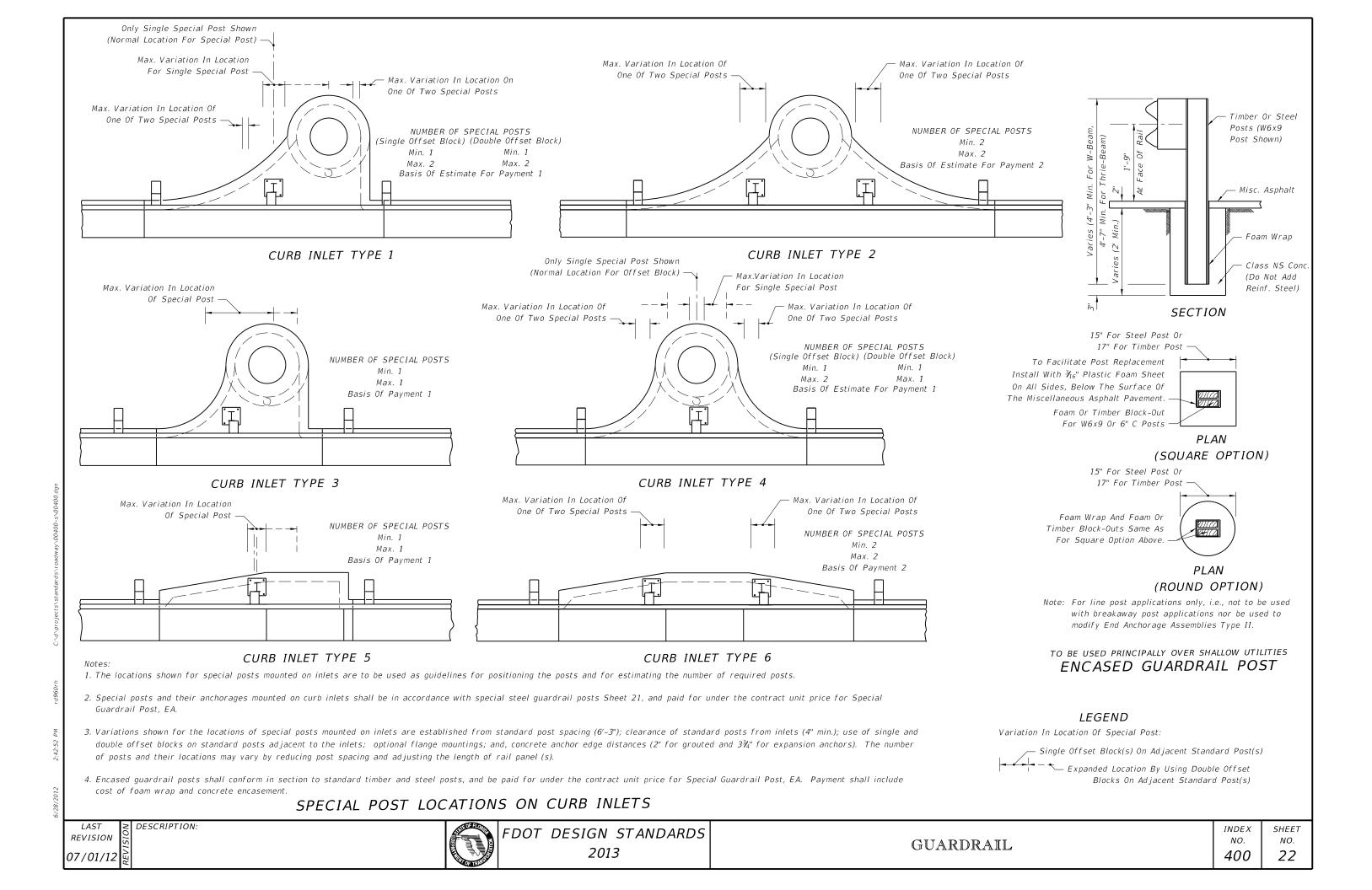
SIDE VIEW FRONT VIEW

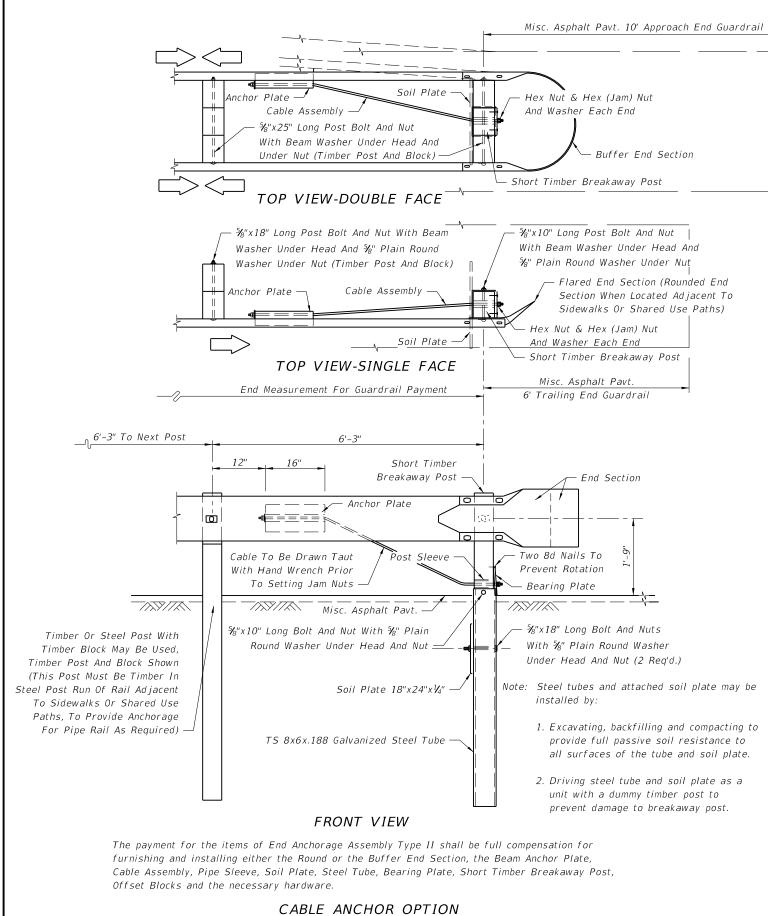
For Use In Combination With

Short Timber Breakaway Post

STEEL TUBE

21





END ANCHORAGE ASSEMBLY TYPE II

Misc. Asphalt Pavt. Misc. Asphalt Pavt. 6' 10' Approach End Guardrail Trailing End Guardrail Approach Rail Buffer End Section For (Position Varies) Approach End Anchorage Steel End Plate, Washer, Hex Nut And Hex (Jam) Nut Anchor Roa Position Varies Trailing Rail Turnbuckle (6") See Note Below Concrete Anchor Block Flared Or Rounded End Section (Block To Be Positioned To Suit On Trailing End Section Anchorage Alignment. Only One TOP VIEW Anchorage Required. Anchorage To Be On Approach Rail When Both Approach And Trailing End Measurement For Guardrail Payment Guardrails Are Connected.) 4'-9" — Standard Post — Anchor Plate Misc. Asphalt Pavt. |-••+••**•**|| ■ | No Cover Required Turnbuckle (6") Misc. Asphalt Pavt. See Note Below 1"Ø Galv Anchor Rod 11/2" ID Pipe Sleeve Galvanized 4"x4"x¾" Plate

Turnbuckle shall be field cut, threaded 4" on each end, and, metalized in accordance with Sections 562 and 975 of the Standard Specifications. The cost for cutting, threading, metalizing and the turnbuckle shall be included in the contract unit price for Reset Guardrail, LF.

FRONT VIEW

The payment for the items of End Anchorage Assembly Type II shall be full compensation for furnishing and installing the Beam Anchor Plate, Anchor Rod, Pipe Sleeve, Anchor Block, either Flared, Rounded or Buffer End Section, and the necessary hardware.

CONCRETE ANCHOR BLOCK OPTION TYPE II NOTES

- 1. Unless specified in the plans, the contractor can supply either the cable anchor option or the concrete anchor block option.
- 2. Type II end anchorage assemblies are approved for all speeds and are intended for use as:
- a. trailing end anchorages for single face free standing guardrail systems;
- b. approach end anchorages for single face free standing guardrail systems when end anchorage is located outside of the
- c. both approach and trailing ends of double face guardrail systems.

Crash cushions shall be constructed at or in lieu of approach Type II end anchorages located inside the clear zone.

End anchorage for thrie beam quardrail shall be constructed the same as detailed for W-beam, except use thrie beam rail and end section; and the Anchor Plate is to be attached to the bottom corrugation of the thrie beam.

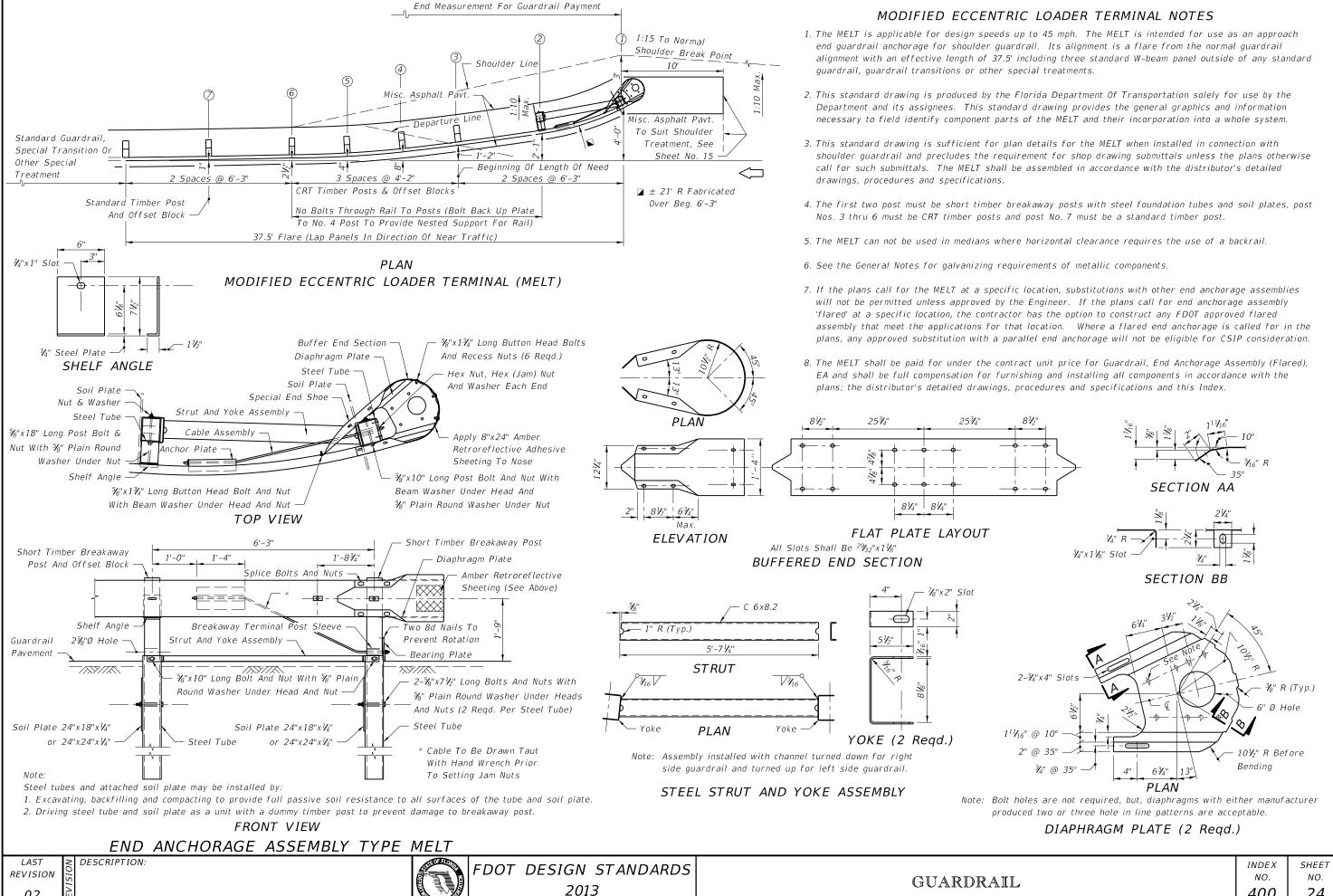
3. These end anchors are to be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Type II), EA as called for in the plans or by permit.

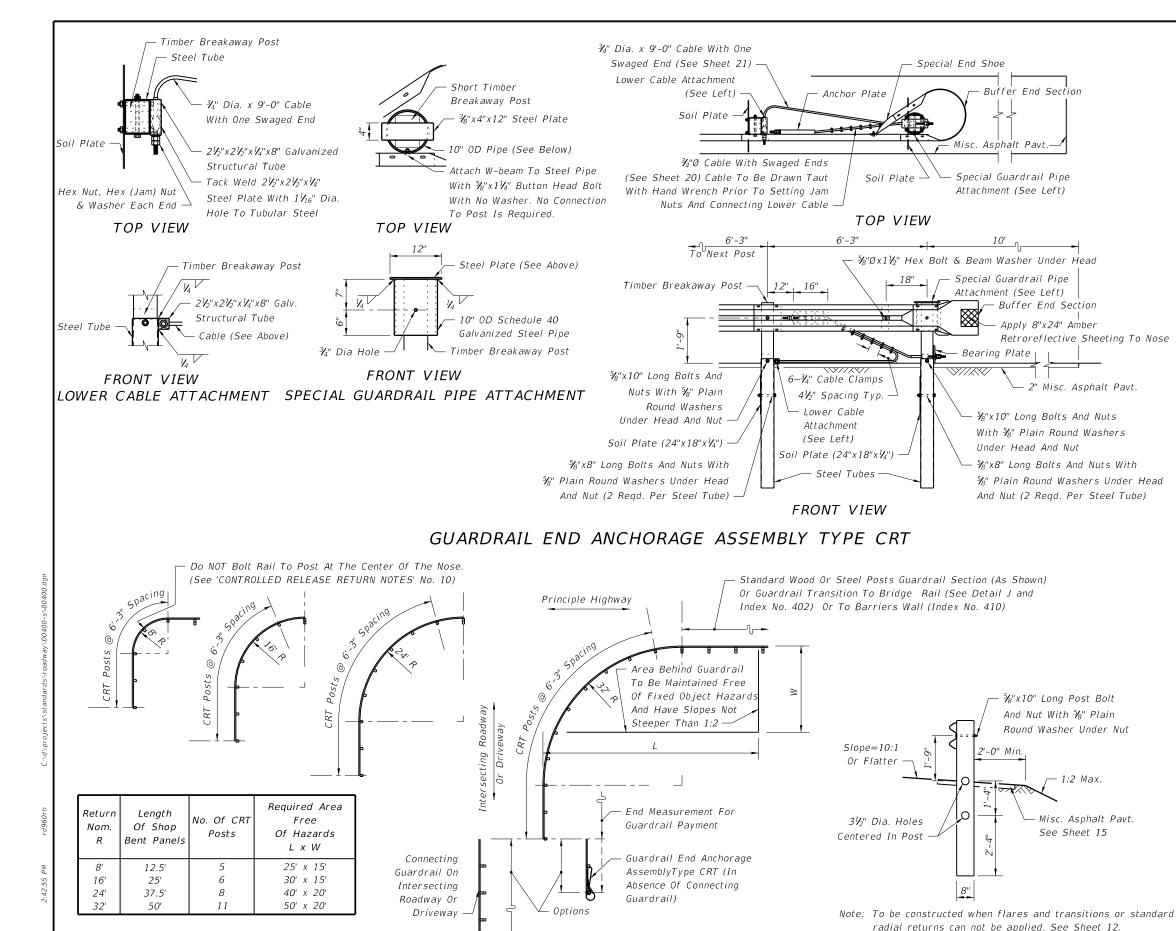
LAST REVISION 07/01/12

DESCRIPTION:



Beveled Washer And Hex Nut





CONTROLLED RELEASE RETURN NOTES

- 1. Controlled release returns are intended for use (a) in openings in continuous guardrail for driveway and side road access when flares and transitions or standard radial returns can not be applied (Sheet 12); and, (b) for shielding the ends of bridge traffic rails and barrier walls where the driveway and side road access is in close proximity to the structure and space does not permit the proper use of approved flared and parallel types of Guardrail End Anchorage Assemblies.
- 2. Controlled release returns are not intended as a substitute or replacement for the appropriate use of approved vehicle impact attenuators.
- 3. Controlled release returns with either 8', 16' or 24' radii are designed for highway speeds of 60 mph or less.
- 4. The controlled release returns shown are designed as full returns based on an intersection angle of 90°. The return can be terminated with the Guardrail End Anchorage Assembly Type CRT or connected to standard quardrail as shown or as otherwise detailed in the plans.
- 5. The Guardrail End Anchorage Assembly Type CRT is to be used only for the controlled release returns with 8', 16', 24' and 32' radii as shown; the assembly is not to be used in any tangent rail or flared rail applications. Other types of end anchorage assemblies are not to be used in the controlled release returns.
- 6. The area immediately behind the control release return shall have slopes not steeper than 1.2 and he maintained free of fixed objects in accordance with the area limits tabulated in the plan below.
- 7. The surface approaching the controlled release return shall have a transverse slope not exceeding 1:10. The effective width of the transverse surface is to be based on standard vehicle departure, return radii and preceding shielding; the width (beyond shoulder) shall be not greater than the corresponding 15' and 20', 'W' values tabulated below.
- 8. The curved quardrail portion of the controlled release return shall be full section shop bent panels (12.5' or 25'
- 9. Washers are not to be used between the guardrail beam and the head of the button head post bolts at any controlled release terminal (CRT) post or at any Guardrail End Anchorage Assembly Type CRT breakaway
- 10. The guardrail beam of the 8' radius return is not bolted to the center control release post.
- 11. See the General Notes for galvanizing requirements of metallic components.
- 12. Controlled release return systems shall be paid for under the contract unit prices for Guardrail (Roadway), LF, Guardrail (Shop-bent Panels), LF, and Guardrail, End Anchorage Assembly (Type CRT), EA as called for in the plans or by permit and shall be full compensation for furnishing and installing all components in accordance with the plans and with this index. CRT posts are included in the cost for guardrail.

CONTROLLED RELEASE RETURN FOR SIDE ROAD AND DRIVEWAY ACCESS

LAST DESCRIPTION: REVISION 04

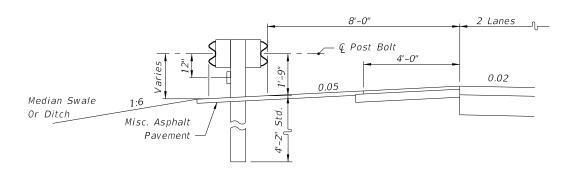


FDOT DESIGN STANDARDS 2013

GUARDRAIL

CRT TIMBER POST

INDEX NO. 400 SHEET NO. 25



Notes:

- 1. Typical placement shown. May be constructed at other locations as called for in the plans.
- 2. Rub Rail is required on the median side or ditch side of the barrier.

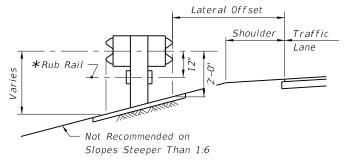
MOUNTING HEIGHT FOR DOUBLE FACED
GUARDRAIL ON MEDIAN SHOULDERS (FREEWAYS)

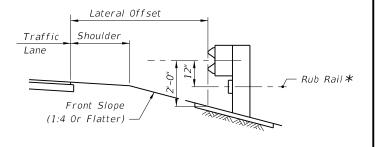
LATERAL PLACEMENT ON SLOPES (FROM EDGE OF NEAR TRAFFIC LANE)¹

Slope	Standard Guardrail ²	Guardrail Not Recommended	Guardrail With Rub Rail ³
1:4	to 13'	14' to 27'	28' to 45'
1:5	to 14'	15' to 25'	26' to 45'
1:6	to 16'	17' to 22'	23' to 45'
1:7	to 20'	21' to 24'	25' to 45'
1:8	to 25'		26' to 45'
1:9	to 26'		27' to 45'
1:10	to 27'		28' to 45'

Notes:

- 1. For shoulders less than 12' in width the tabulated values will be reduced by the difference between 12' and the shoulder width. Placement of guardrail on front slopes steeper than 1:4 not recommended.
- 2. Standard guardrail; 1'-9" to @ post bolt. Rub Rail is required on the median side when double face guardrail is used.
- 3. Guardrail with Rub Rail; 2'-0" to & post bolt.



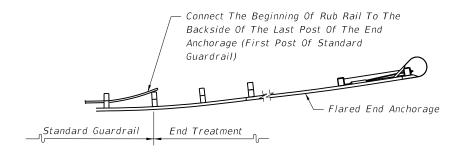


GUARDRAIL ON MEDIAN SLOPES

GUARDRAIL ON OUTSIDE SLOPES

* C6x8.2, Plates And Fasteners or Bent Plate And Fasteners In Accordance With Standards RLR01 And RER01 Of AASHTO-AGC-ARTBA "A Guide To Standardized Highway Barrier Hardware"

GUARDRAIL ON SLOPES



RUB RAIL TERMINATION

LAST REVISION 07/01/12

DESCRIPTION:



FDOT DESIGN STANDARDS
2013

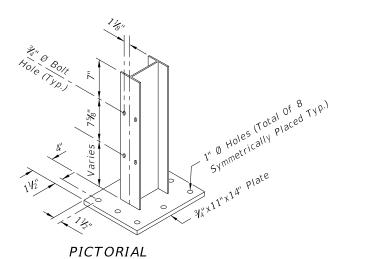
GUARDRAIL

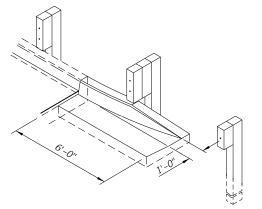
INDEX NO. 400

SHEET NO. **26**

N STANDARDS

07/01/07



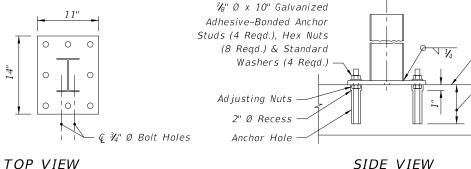


CURB TYPE F FLARE WHEN END OF EXISTING APPROACH SLAB CURB EXPOSED

Remove Any Asphalt To Set

Base Plate Flush With Slab

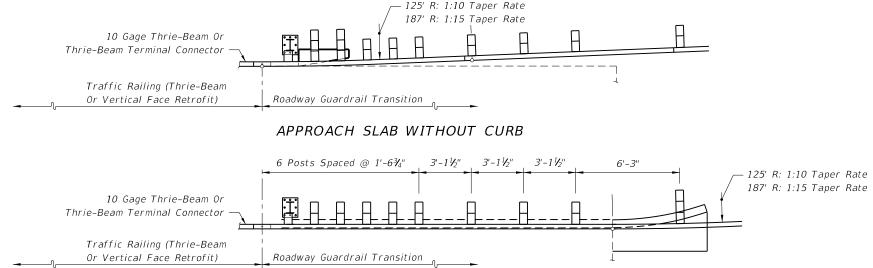
7½" (Max.)



SIDE VIEW

€ W6x9 Post

SPECIAL STEEL POST FOR ROADWAY THRIE-BEAM TRANSITIONS TO BRIDGE TRAFFIC RAILING RETROFITS



APPROACH SLAB WITH CURB

Longitudinal Location Of Transition Blocks And Curb End Flares Will Vary With Scheme Type

PARTIAL PLAN VIEWS

GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT



GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES

INDEX NO.

402

DESCRIPTION: REVISION

NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

GENERAL NOTES 1. This index provides thrie-beam transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 23 apply to bridges with retrofitted traffic railings, (Sheet 23 shows the trailing end quardrail connections). Sheet 24 applies to bridges

2. The schemes identified by Arabic numerals in this index are complementary to the bridge traffic railing barrier retrofit schemes with like numeral identification in Index Nos. 470, 471 through 476, 480 through 483. The schemes in this index identified by Roman numerals are

with safety shaped traffic railing.

- 1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.
- 2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to curb blunt ends are not in place.
- 3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated using ASTM A36 or ASTM A709 Grade 36 steel. Welding shall conform to ANSI/AASHTO/AWS D1.5. The assembly shall be hot-dip zinc coated in accordance with Section 536 of the Specifications.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

- 5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 12 through 15 and the flag notation on Sheet 23.
- 6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

DESIGN NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

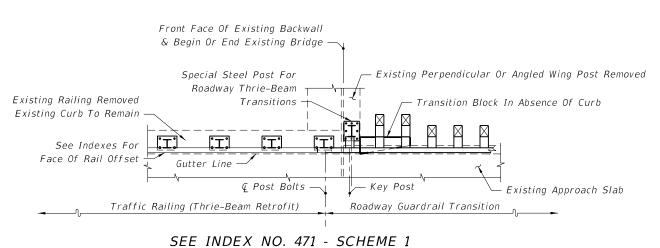
1. For selection of an appropriate transition scheme, see the Structures Manual for instructions to the Structures and Roadway engineers.

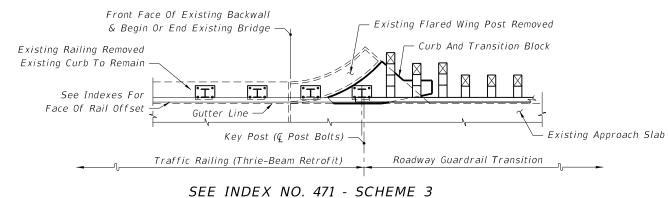
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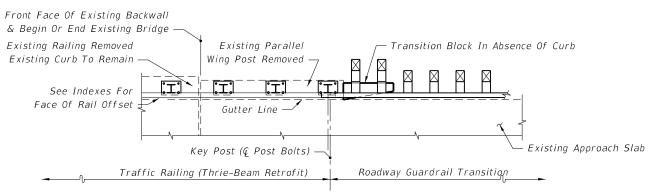
REVISION

07/01/07

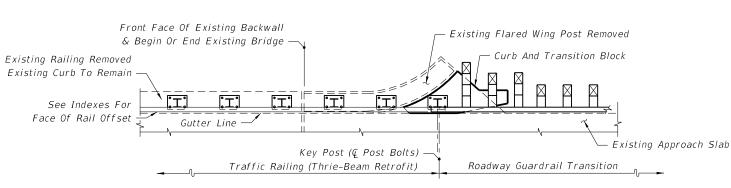








SEE INDEX NO. 471 - SCHEME 2



SEE INDEX NO. 471 - SCHEME 3

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

Front Face Of Existing Backwall

& Begin Or End Existing Bridge

II.

Traffic Railing (Thrie-Beam Retrofit)

Gutter Line -

€ Post Bolts

SEE INDEX NOS. 472 & 475 - SCHEME 1

Existing Curb

DESCRIPTION:

See Indexes For

Face Of Rail Offset

LAST

REVISION

07/01/07

Existing Perpendicular Or Angled Wing Post

Tra<u>n</u>sition Block In Absence Of Curb

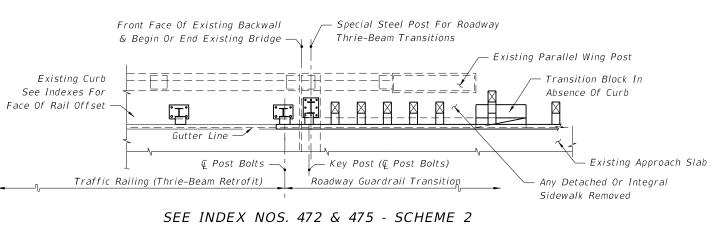
└─ Existing Approach Slab

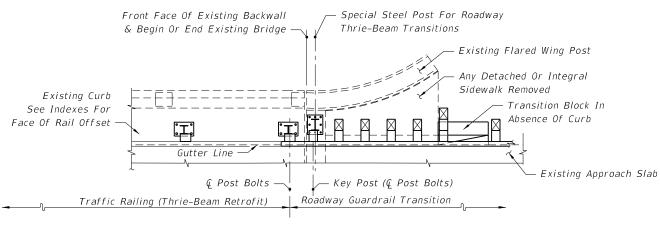
Special Steel Post For Roadway

Thrie-Beam Transitions

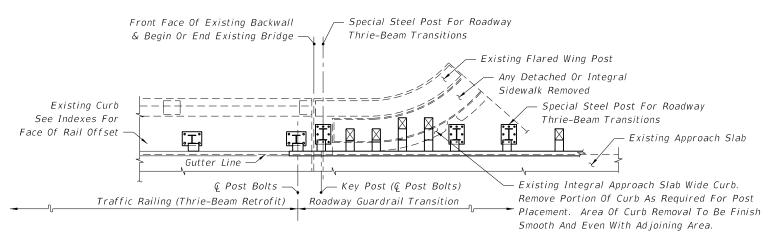
— Key Post (ℚ Post Bolts)

Roadway Guardrail Transition





SEE INDEX NOS. 472 & 475 - SCHEME 2



SEE INDEX NOS. 472 & 475 - SCHEME 2

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

LAST REVISION 07/01/07

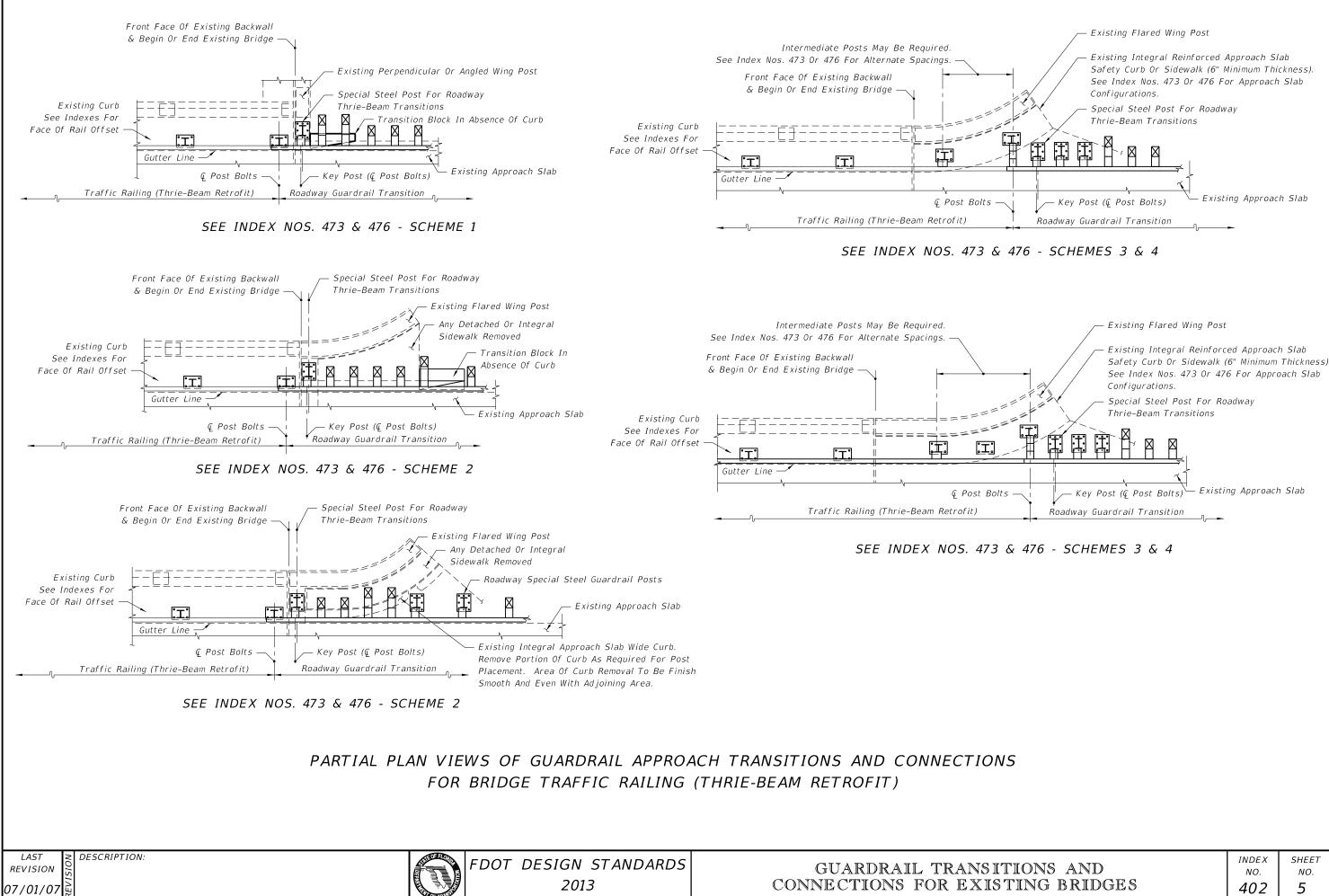
DESCRIPTION:



Existing Approach Slab

Transition Block In Absence Of Curb

Existing Approach Slab





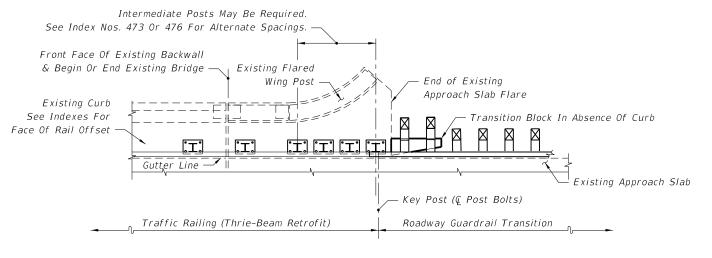
LAST

REVISION

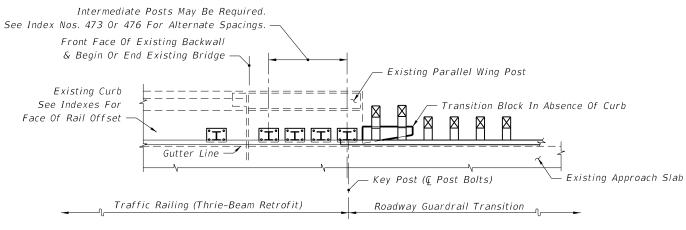
07/01/07

DESCRIPTION:





SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6



SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

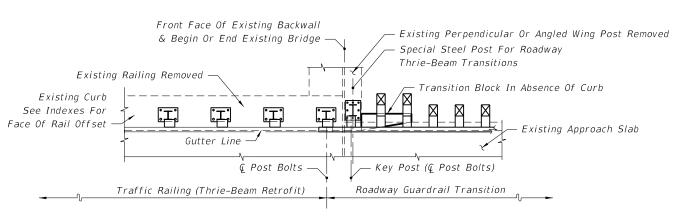
2013

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

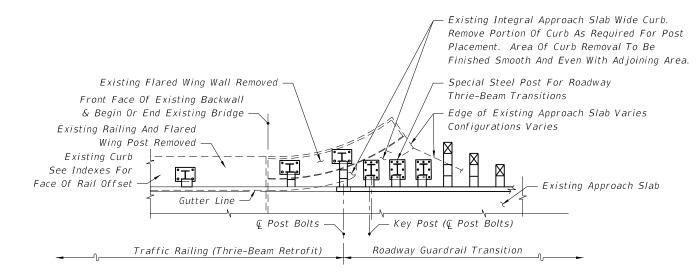
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REVISION

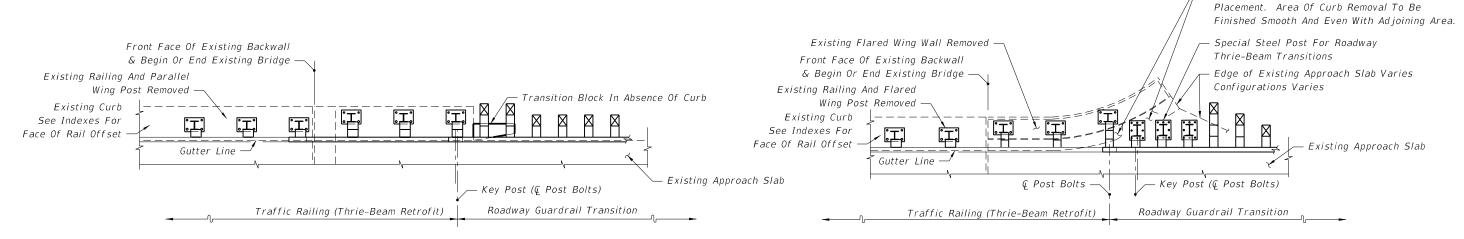
07/01/07



SEE INDEX NO. 474 - SCHEME 1



SEE INDEX NO. 474 - SCHEME 3



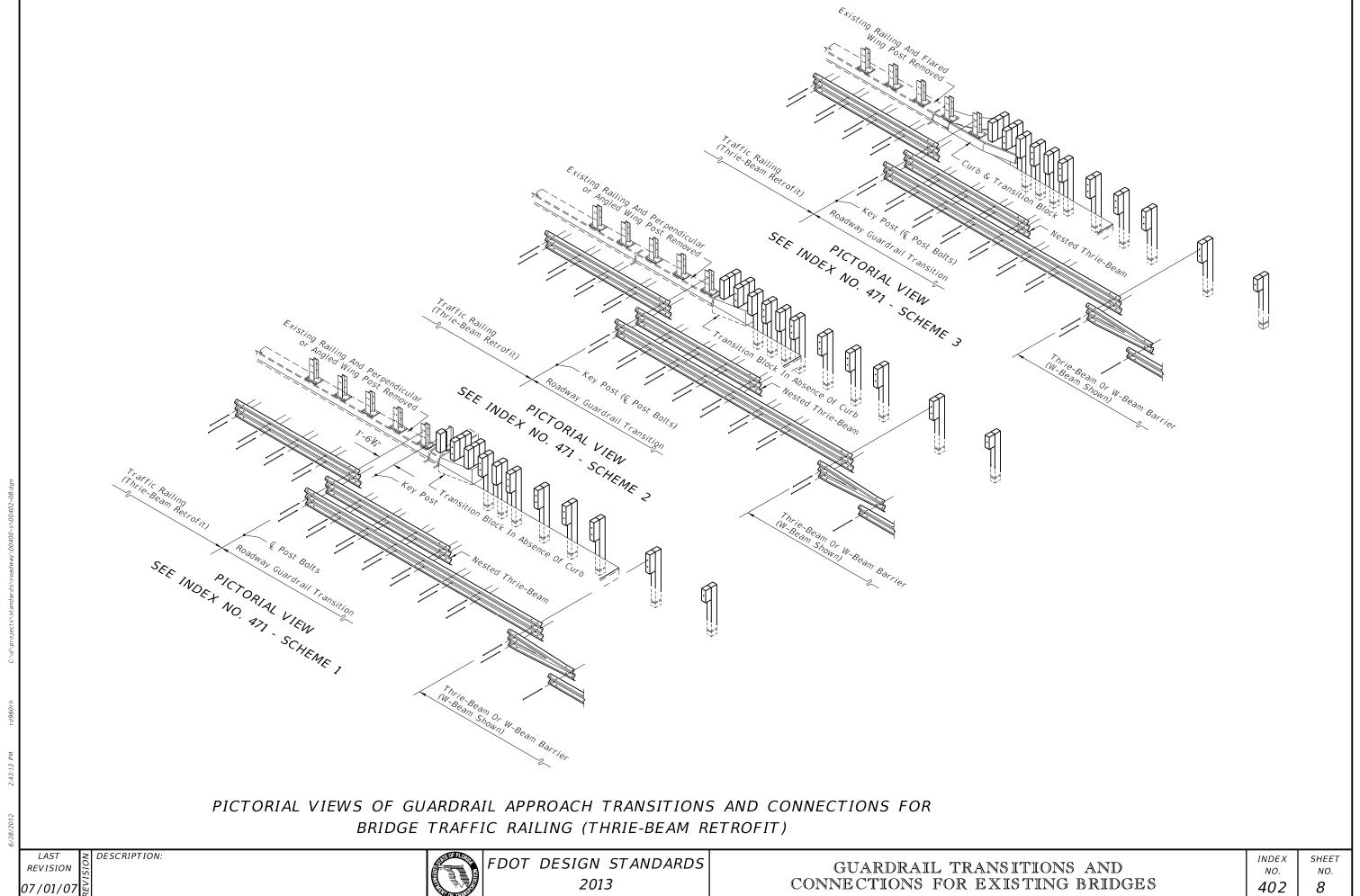
SEE INDEX NO. 474 - SCHEME 2

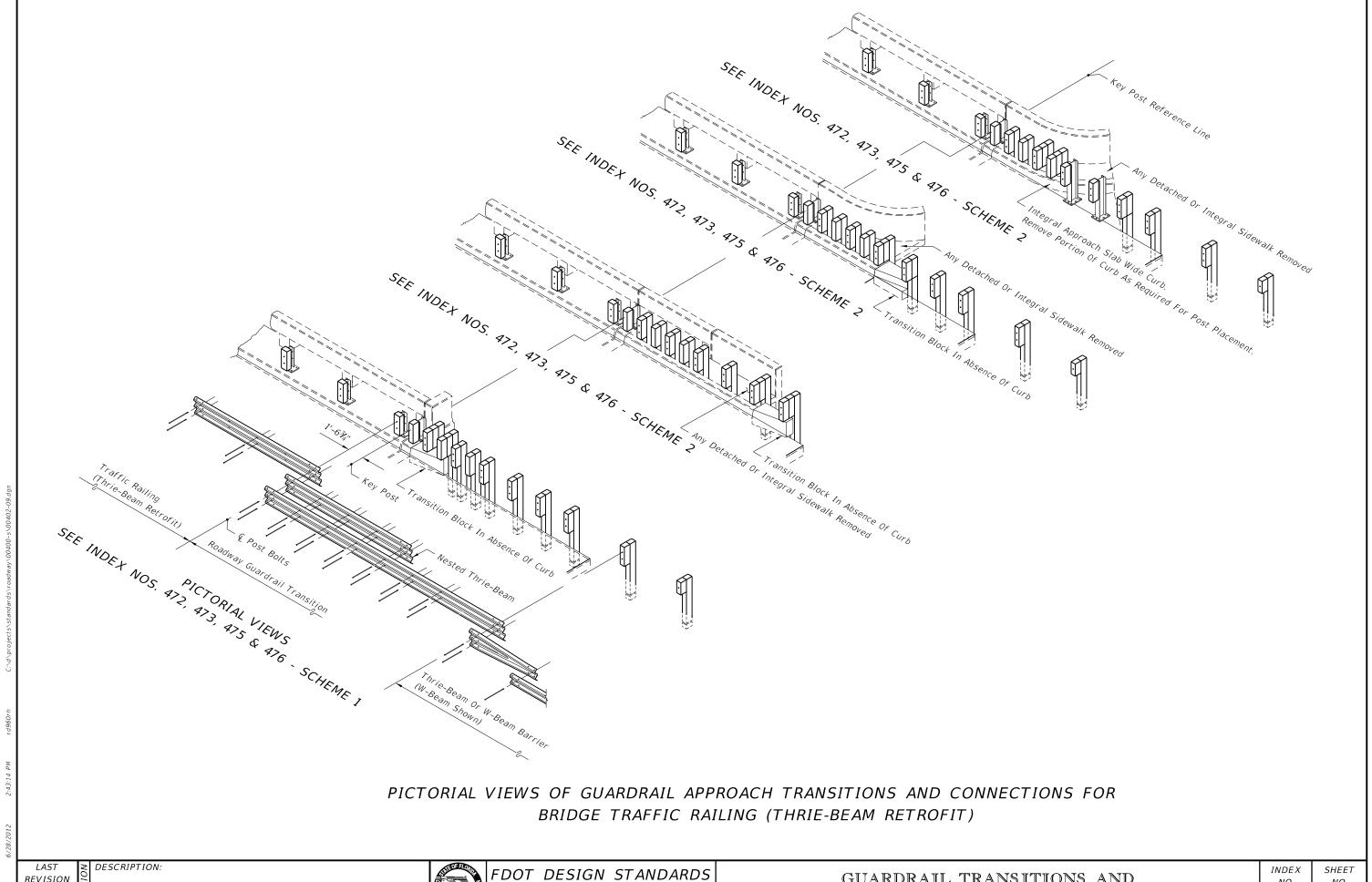
SEE INDEX NO. 474 - SCHEME 3

PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

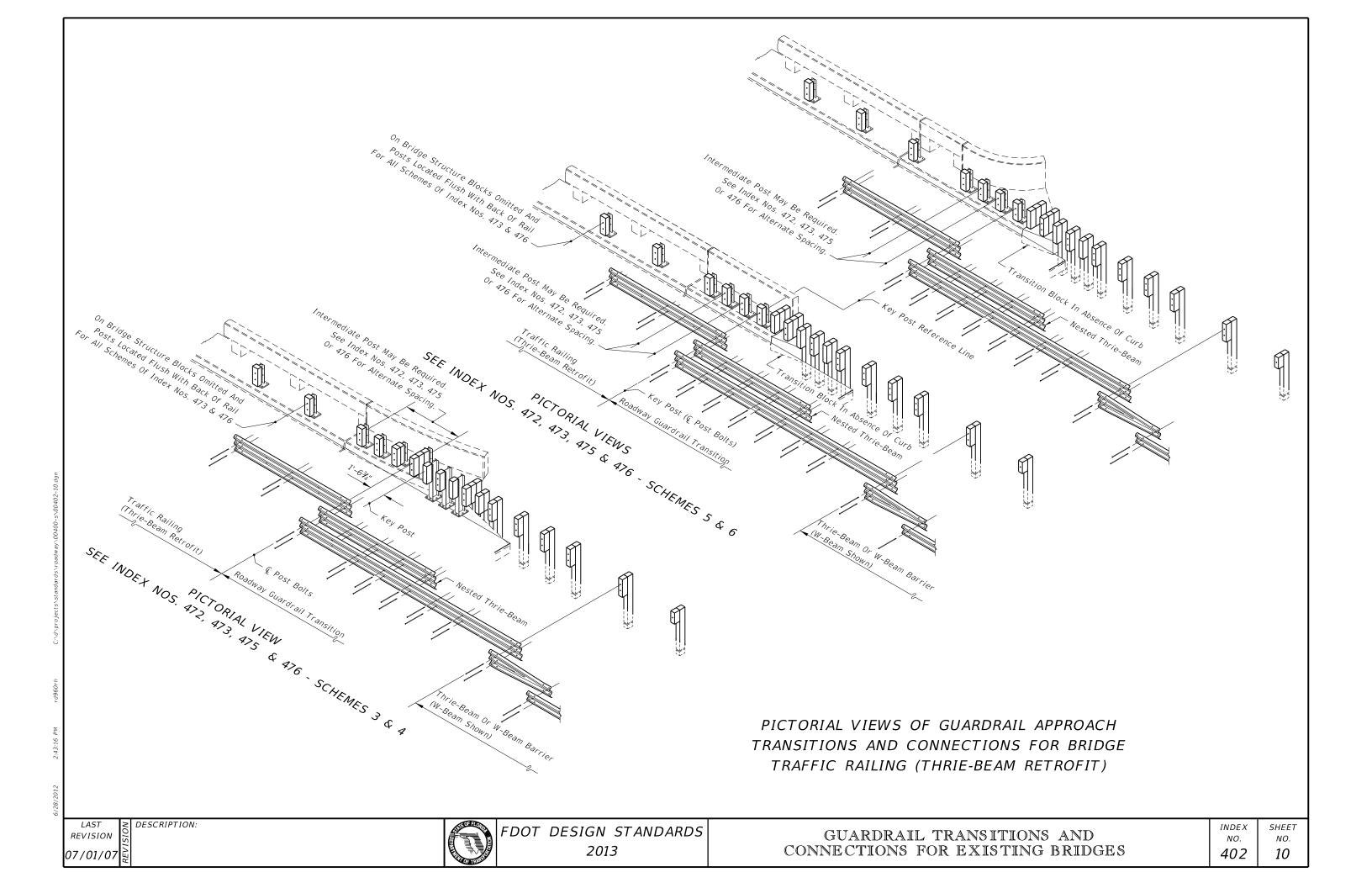


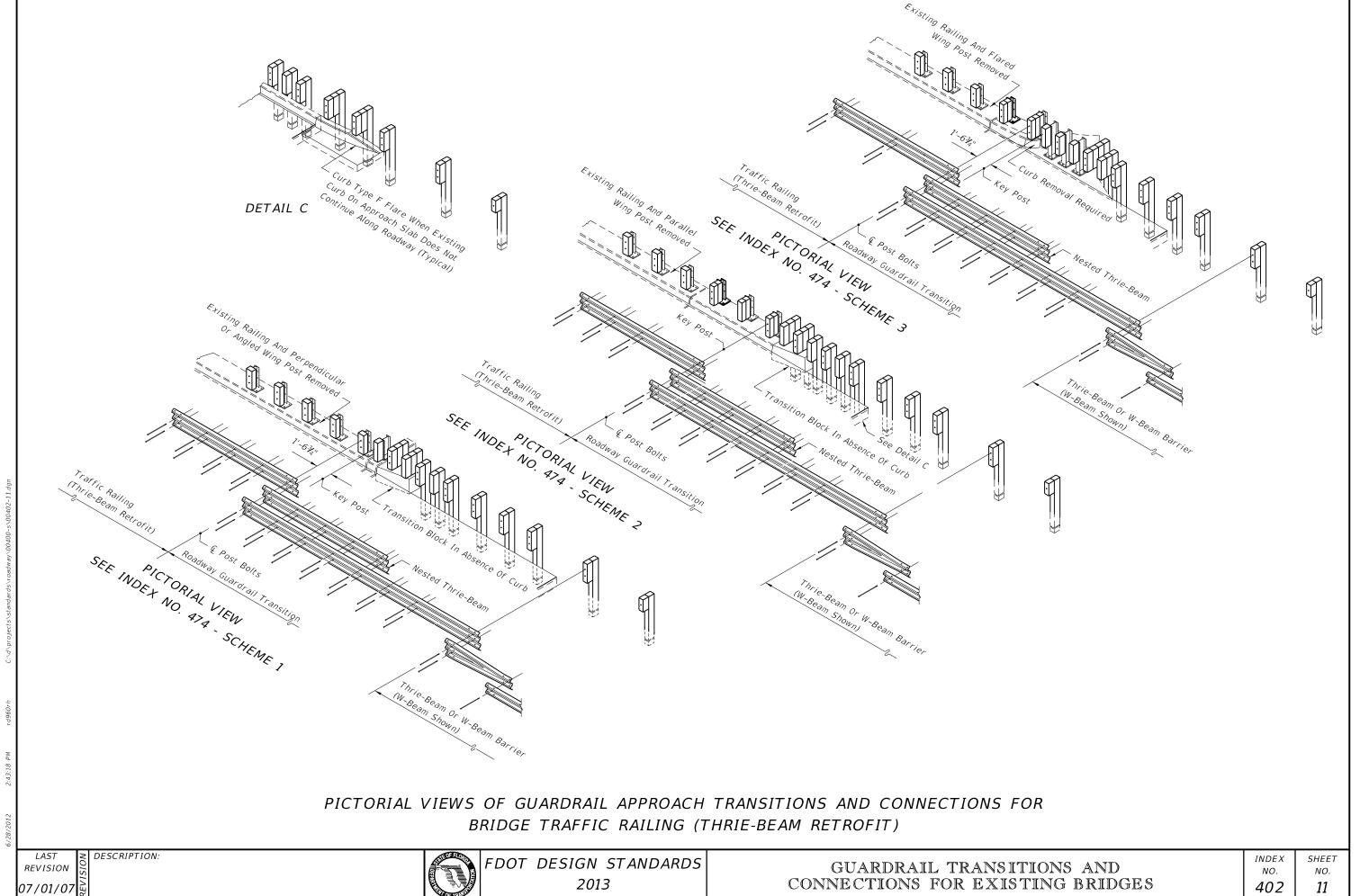
Existing Integral Approach Slab Wide Curb. Remove Portion Of Curb As Required For Post





REVISION 07/01/07





07/01/07



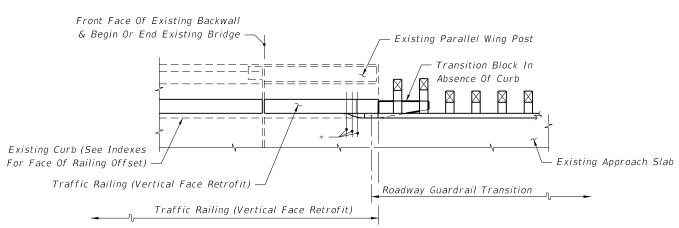
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REVISION

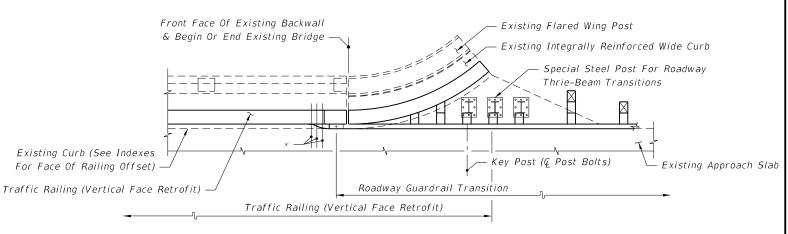
07/01/07

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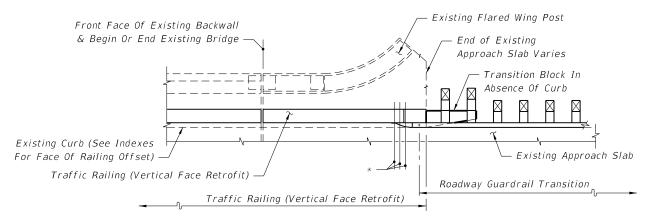




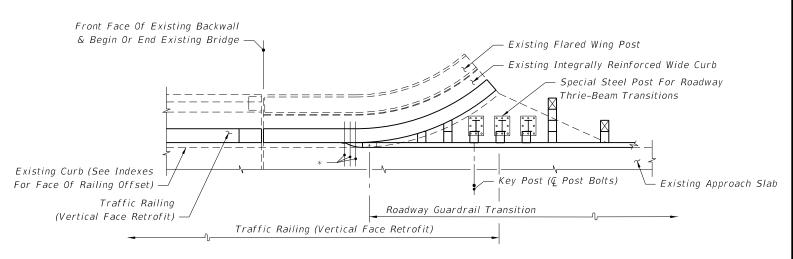
SEE INDEX NO. 482 - SCHEME 2



SEE INDEX NO. 482- SCHEME 3



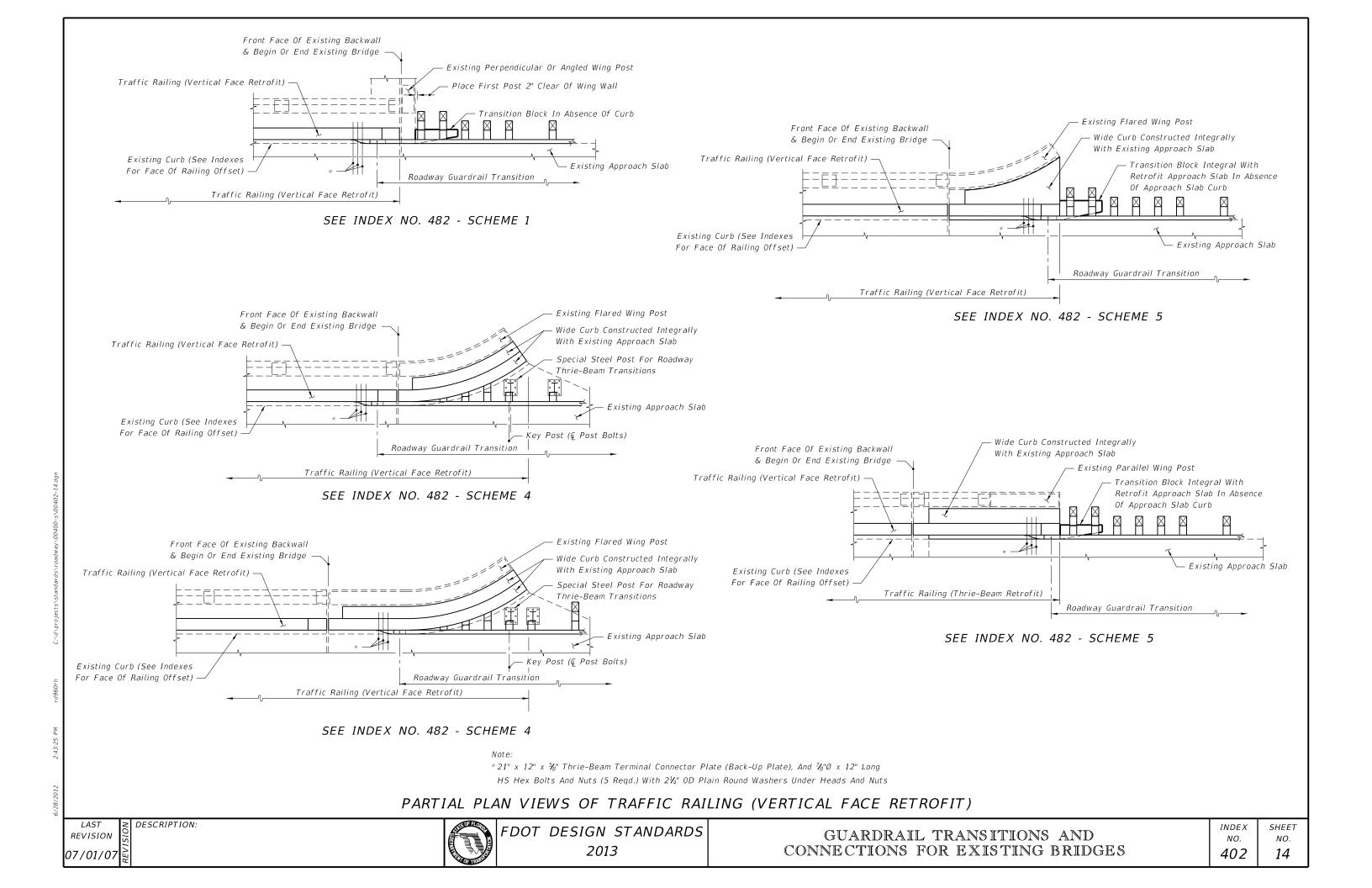
SEE INDEX NO. 482 - SCHEME 2



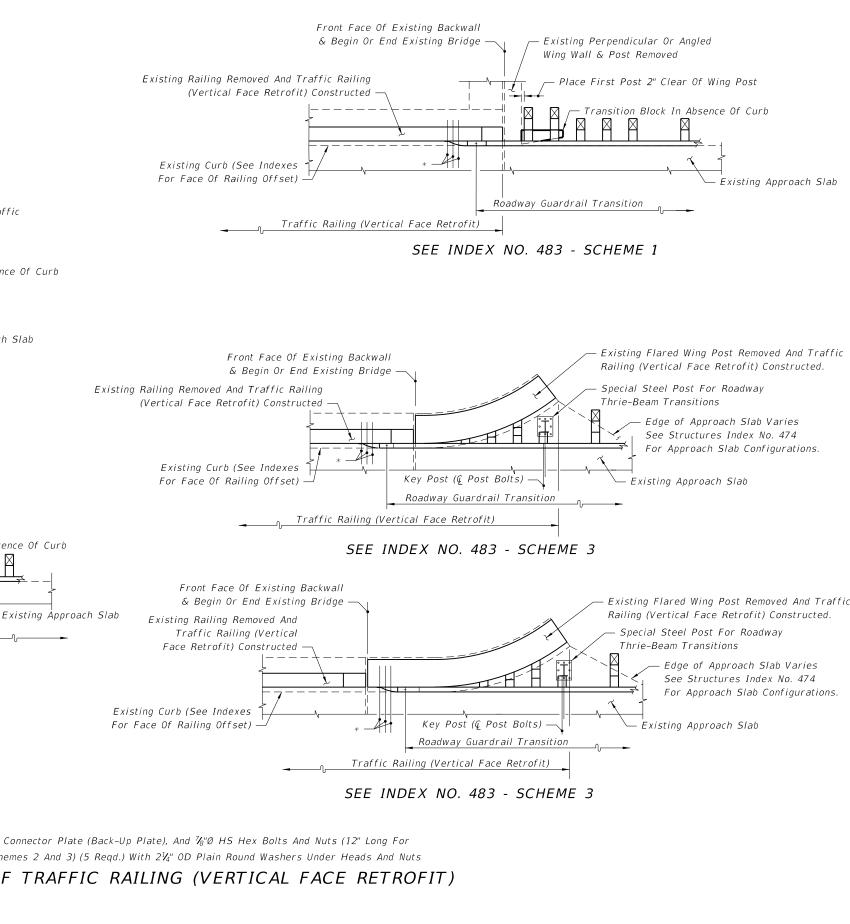
SEE INDEX NO. 482- SCHEME 3

*21" x 12" x $lac{N_0}{8}$ " Thrie-Beam Terminal Connector Plate (Back-Up Plate), And $rac{N_0}{6}$ " \emptyset x 12" Long HS Hex Bolts And Nuts (5 Reqd.) With $2 {\cal V}_4$ " OD Plain Round Washers Under Heads And Nuts

PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)







SEE INDEX NO. 483 - SCHEME 2

Roadway Guardrail Transition

SEE INDEX NO. 483 - SCHEME 2

Existing Parallel Wing Post Removed and Traffic

Railing (Vertical Face Retrofit) Constructed

- Existing Parallel Wing Post Removed and Traffic

Transition Block In Absence Of Curb

Existing Approach Slab

Transition Block In Absence Of Curb

Roadway Guardrail Transition

Railing (Vertical Face Retrofit) Constructed

Front Face Of Existing Backwall

Existing Railing Removed

Traffic Railing (Vertical

Face Retrofit) Constructed

Existing Curb (See Indexes For Face Of Railing Offset)

Front Face Of Existing Backwall

& Begin Or End Existing Bridge

Existing Railing Removed

Traffic Railing (Vertical

Face Retrofit) Constructed

Existing Curb (See Indexes

For Face Of Railing Offset)

& Begin Or End Existing Bridge -

Traffic Railing (Vertical Face Retrofit)

Traffic Railing (Vertical Face Retrofit)

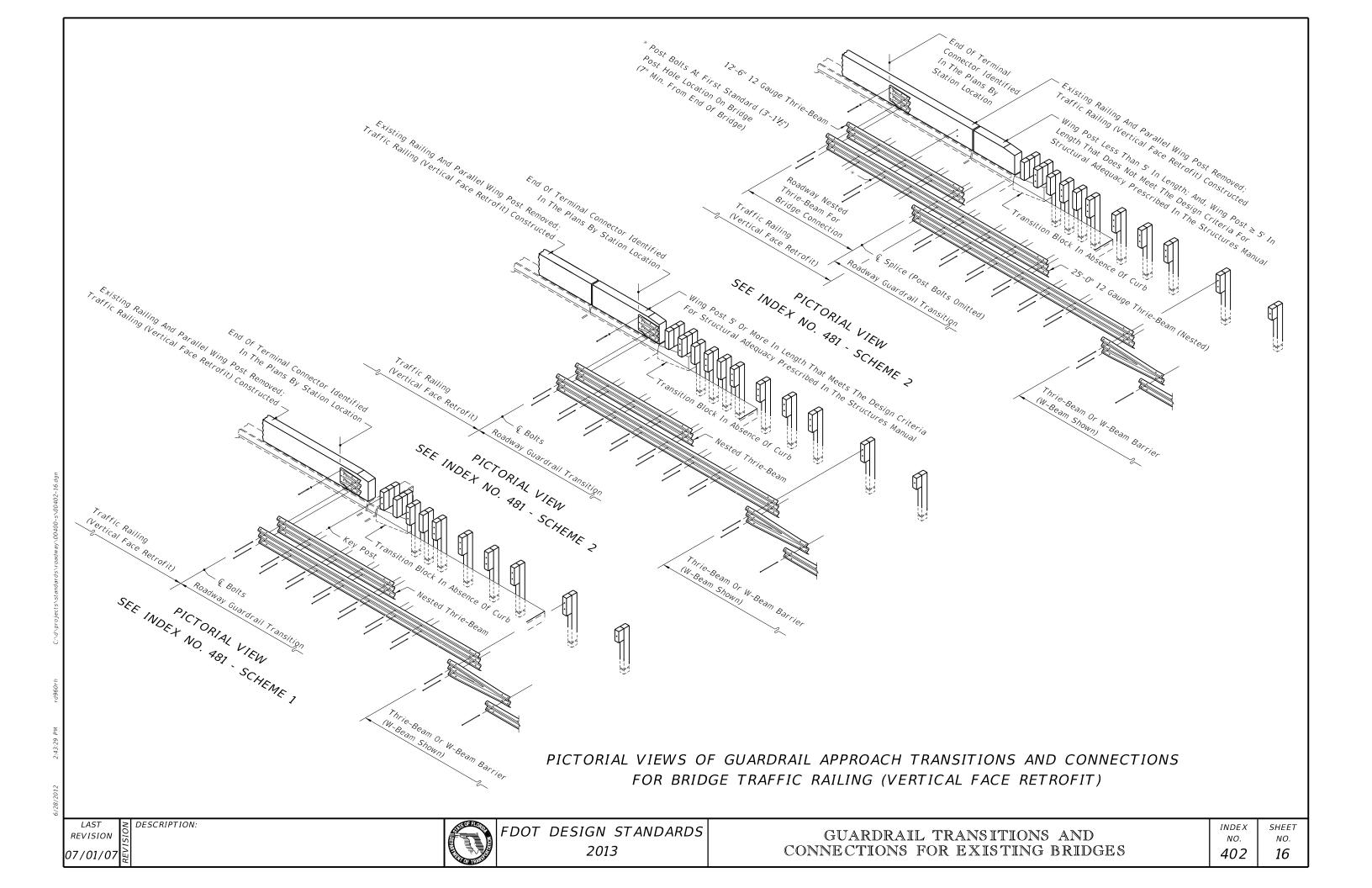
*21" x 12" x 🍇" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And 🔏 "Ø HS Hex Bolts And Nuts (12" Long For Scheme 1 And Length To Fit For Schemes 2 And 3) (5 Reqd.) With 2½" OD Plain Round Washers Under Heads And Nuts

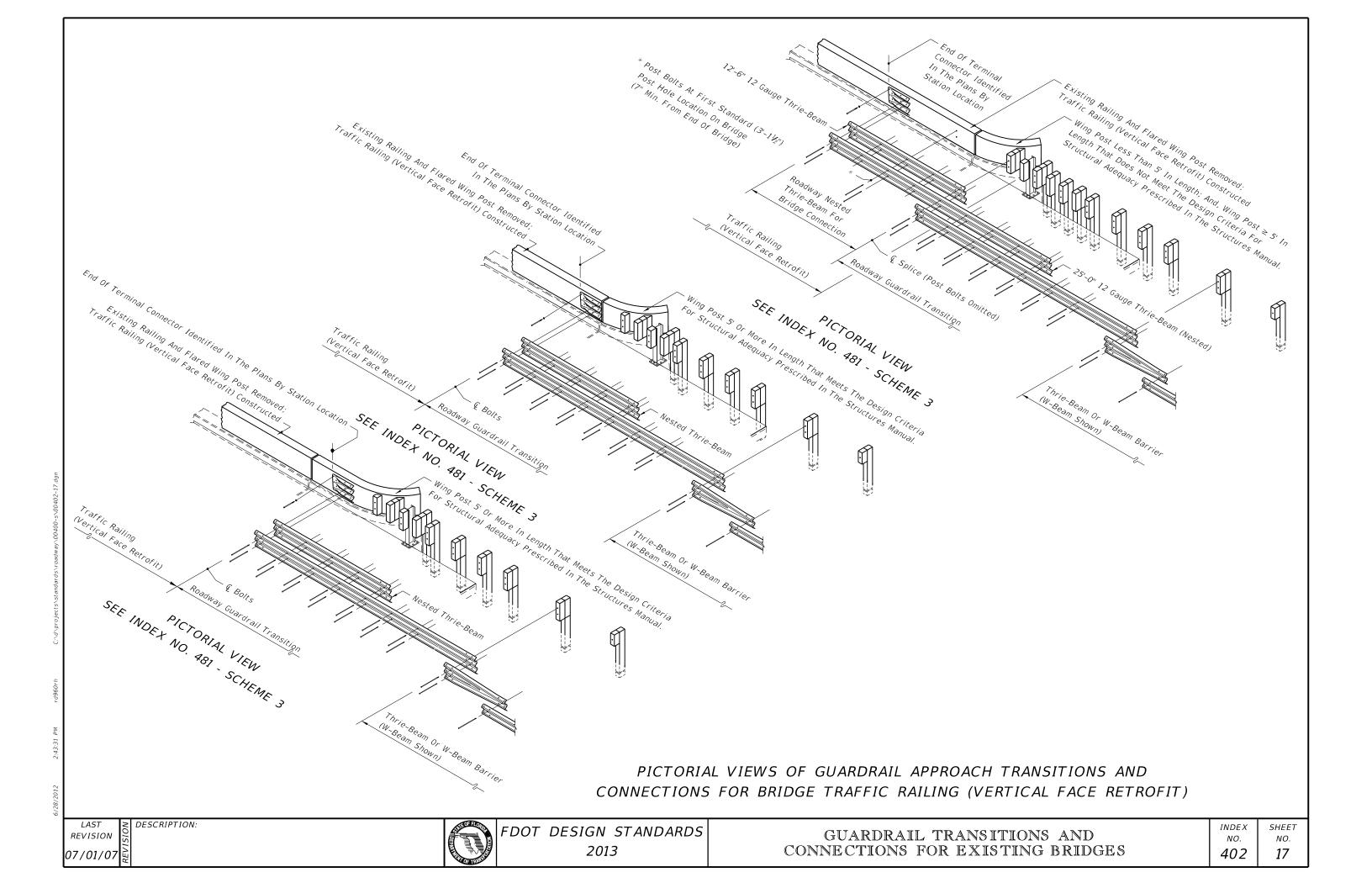
2013

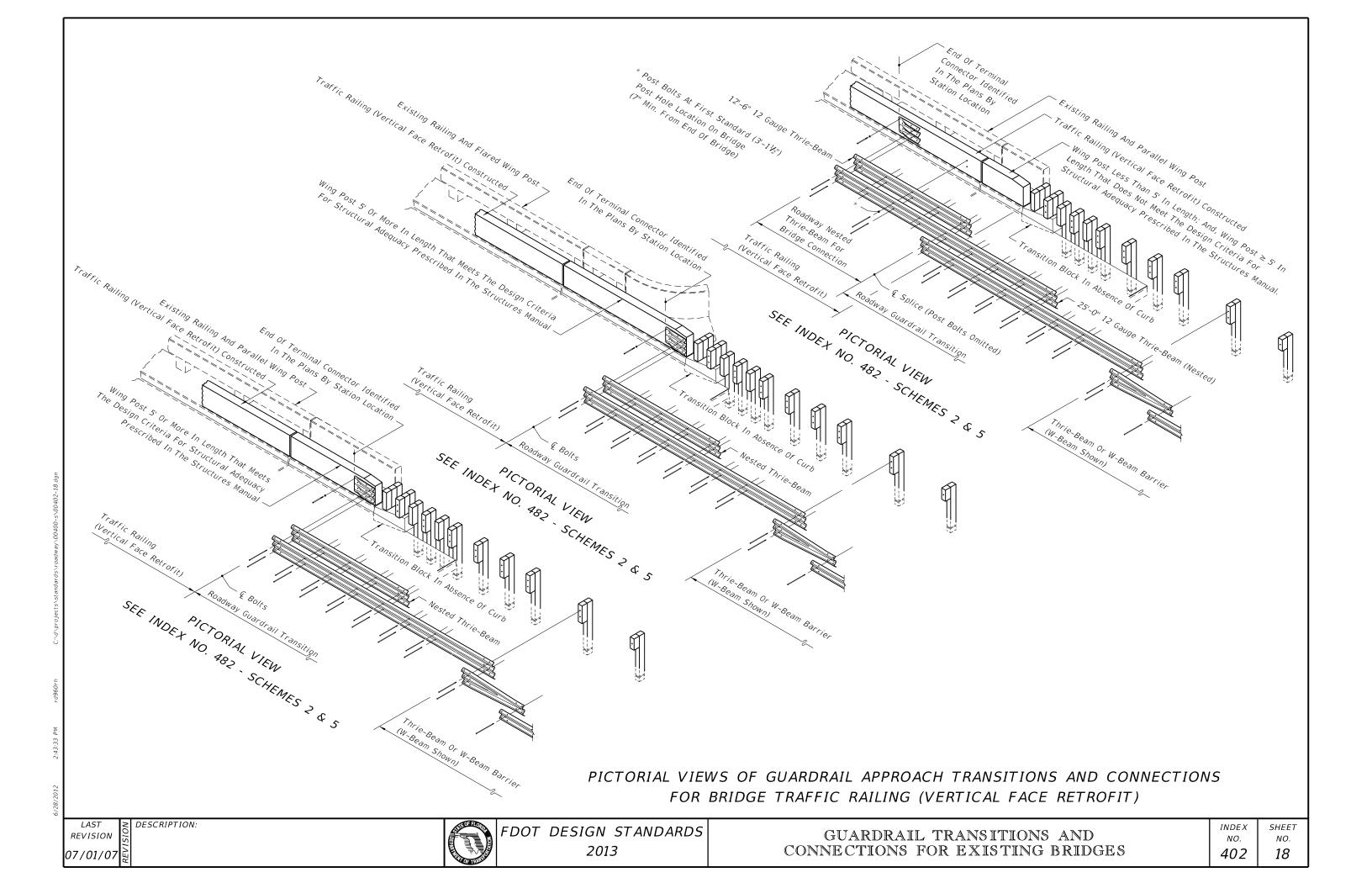
PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)

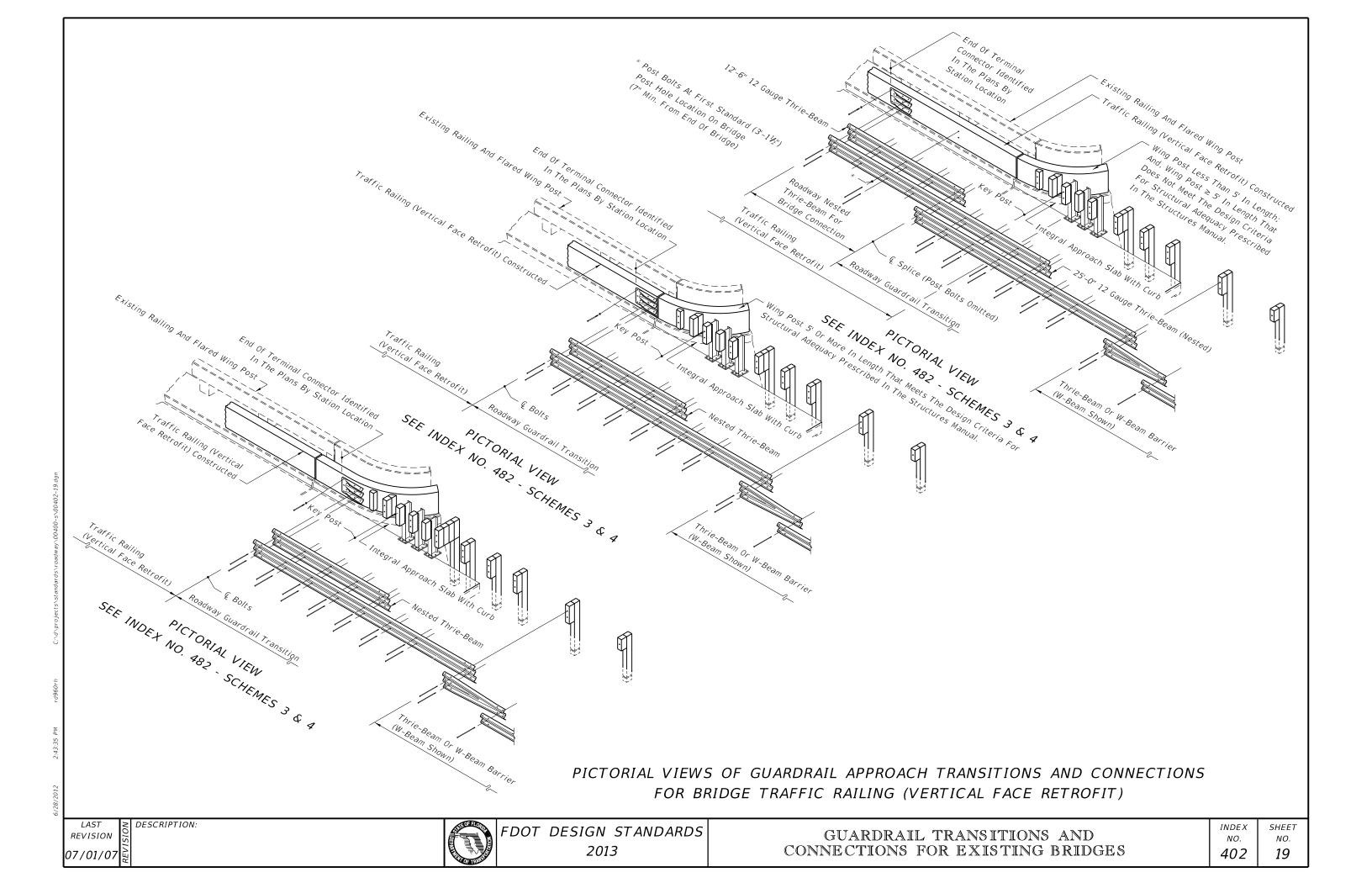
FDOT DESIGN STANDARDS

07/01/07



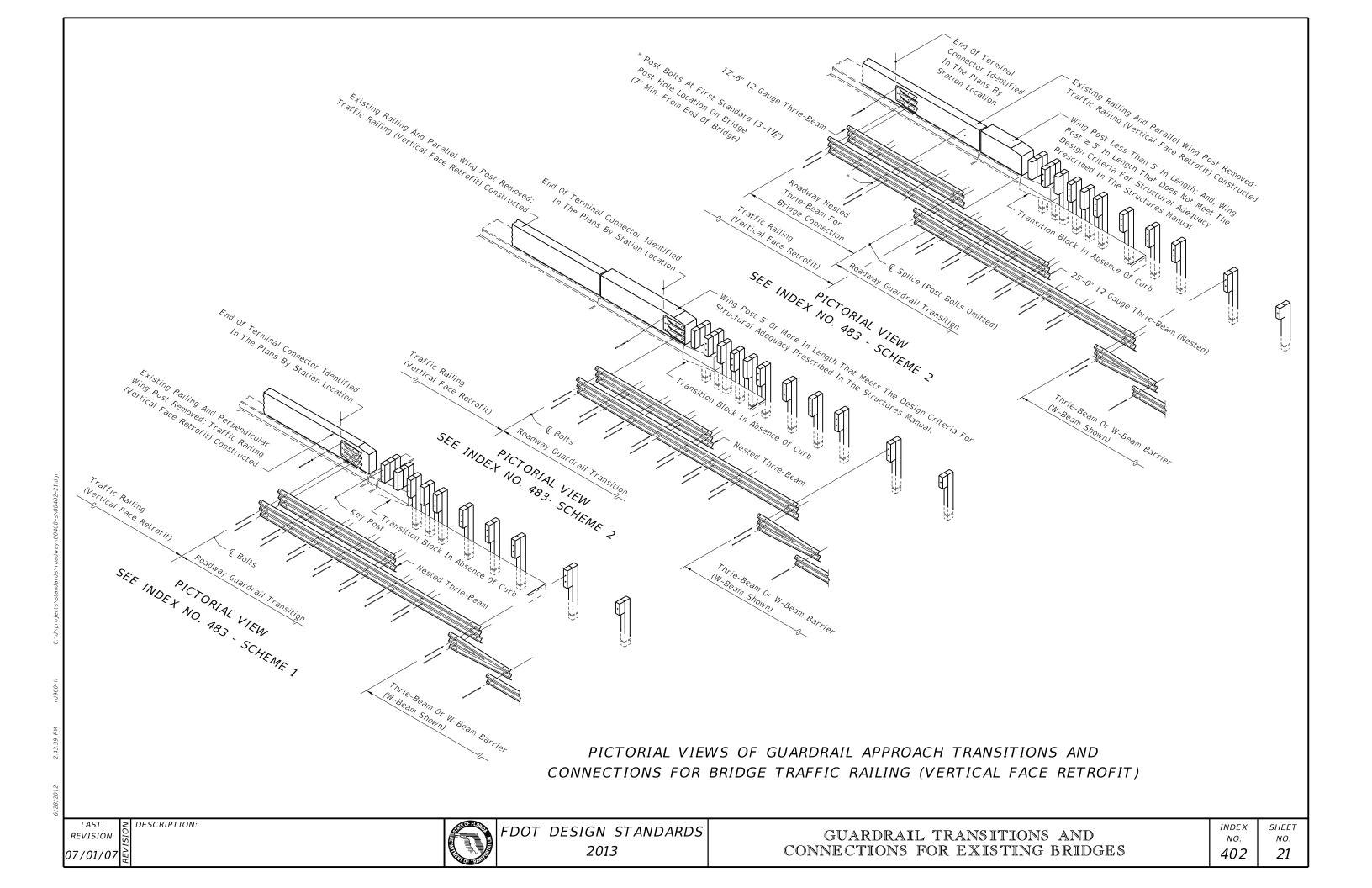


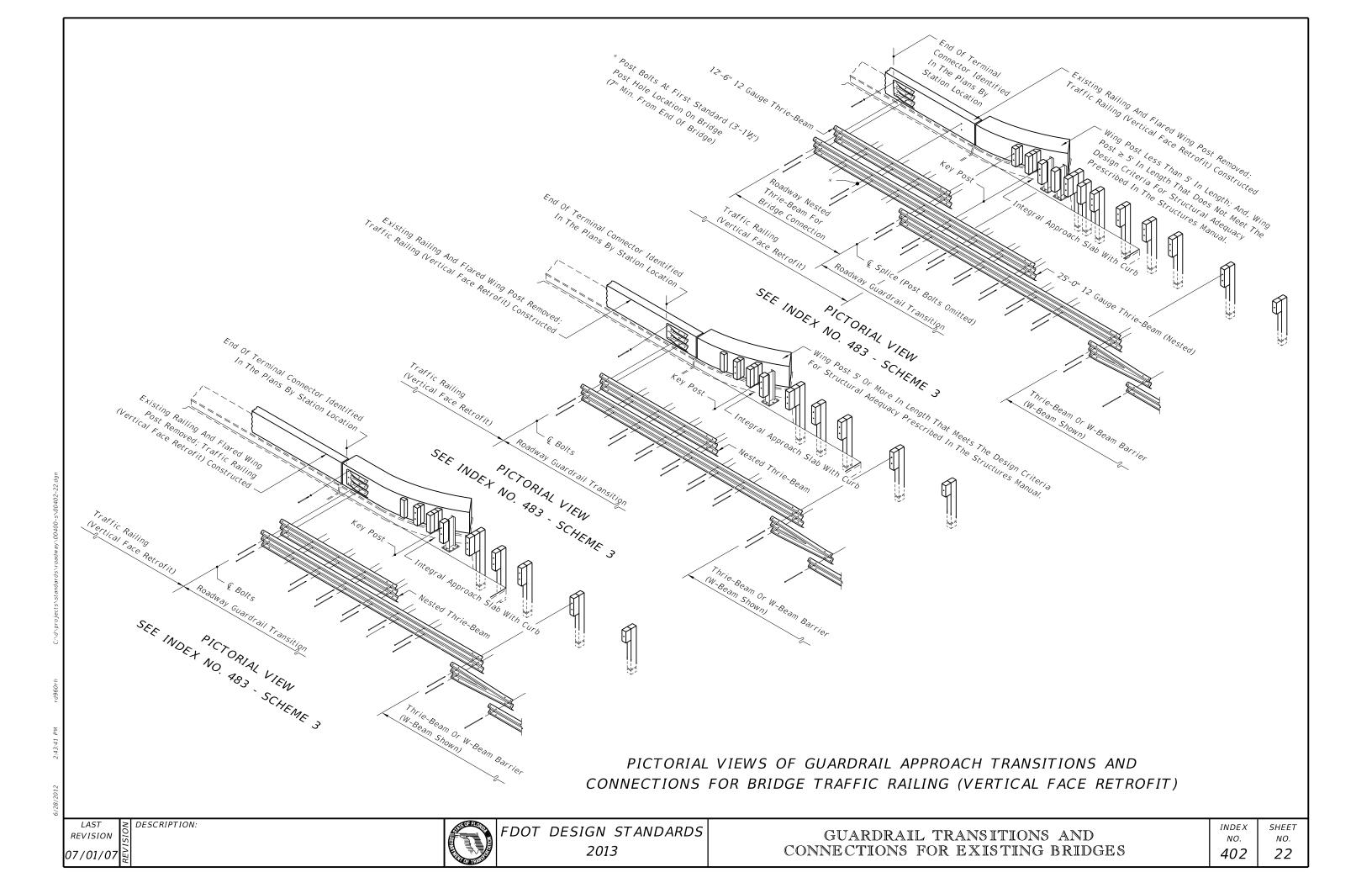




PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT)

LAST REVISION

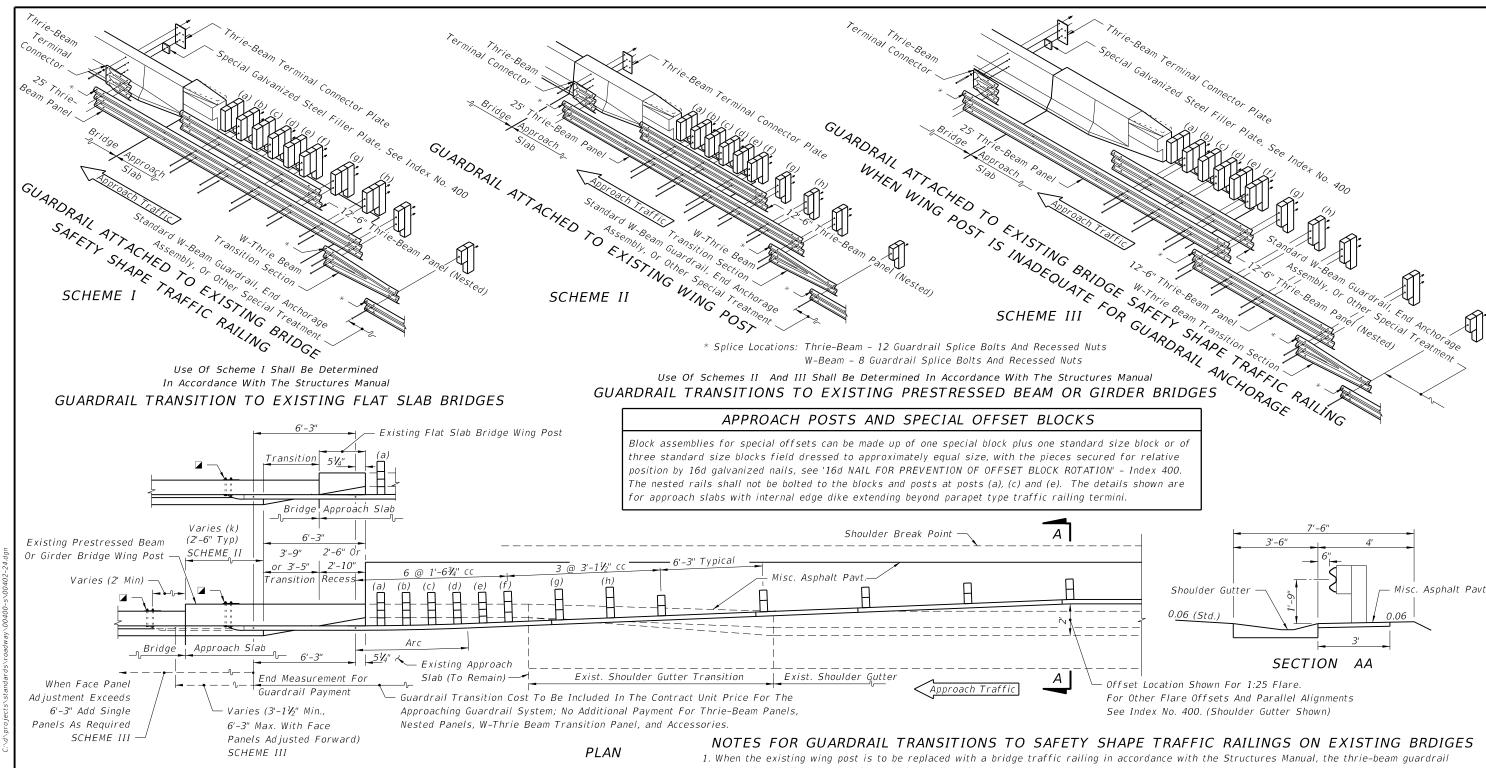




DESCRIPTION:

TRAILING END GUARDRAIL AND ANCHORAGE FOR BRIDGE TRAFFIC RAILING (THRIE BEAM RETROFITS)

LAST REVISION 07/01/07



☑ 21"x12"x⅓" Thrie-Beam Terminal Connector Plate (Back-Up Plate), And ⅙"∅ x 18" Long [15" Long With 3½" Min. Thread Length For Bridge Safety Shape Railing] HS Hex Bolts And Nuts (5 Regd.) With 2½" OD Plain Round Washers Under Heads And Nuts. [When Attaching Guardrail To Existing Wing Posts Or Bridge Rails, Care Should Be Exercised To Avoid Damaging Conduits And Their Utilities That May Be Routed Through Wing Posts Or Bridge Rails. When Conduits And Their Utilities Are Encountered, At Least Five 1/2" HS Hex Bolts Shall Be Installed In Any Of The Seven Holes Provided In The Thrie-Beam Terminal Connector.]

- connection shall be in accordance with Detail J of Index No. 400.
- 2. When the guardrail attachment overlays the Bridge Number, Bridge Name or Date on the traffic railing, provide an aluminum sign panel with the obscured information. Attach the sign panel to the face of the traffic railing adjacent to the Thrie-Beam Terminal Connector with 1/4"0 x 1" long concrete screws or expansion anchors at each corner, as approved by the Engineer. The sign panel shall be a minimum y_{16} " thick and meet the requirements of Specification Section 700 with a white background and 3" tall black letters and sized appropriately to contain the information required. The cost of the sign panel shall be included in the cost of the Guardrail Bridge Anchorage Assembly.
- 3. When retrofitting thrie-beam quardrail to existing wing posts or existing bridge safety shape traffic railing, attachment construction to be paid for under the contract unit price for Guardrail Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate(s) and bolts, nuts and washers.

GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR EXISTING FLAT SLAB, PRESTRESSED BEAM AND GIRDER BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH

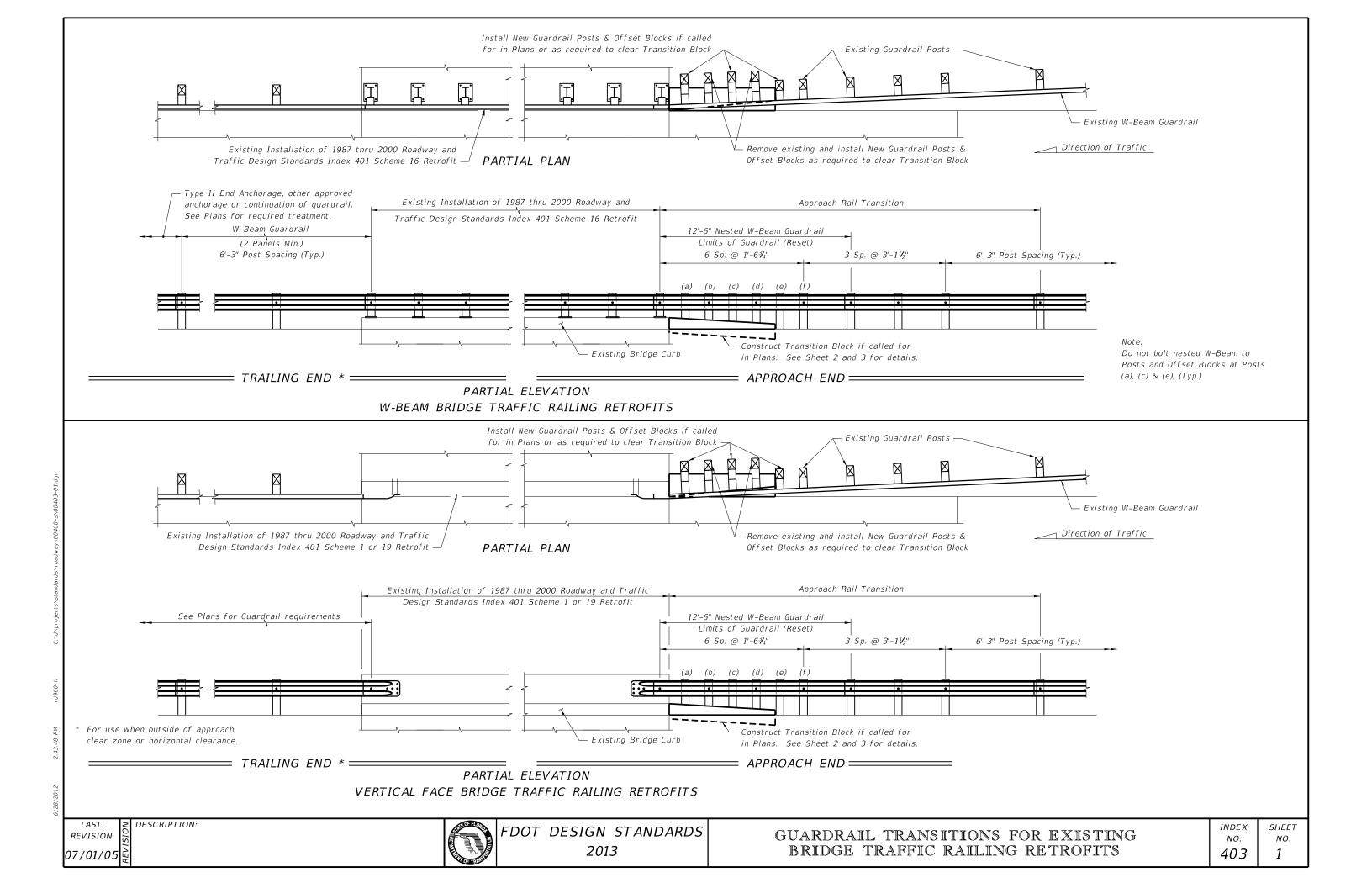
LAST REVISION 01/01/10

DESCRIPTION.



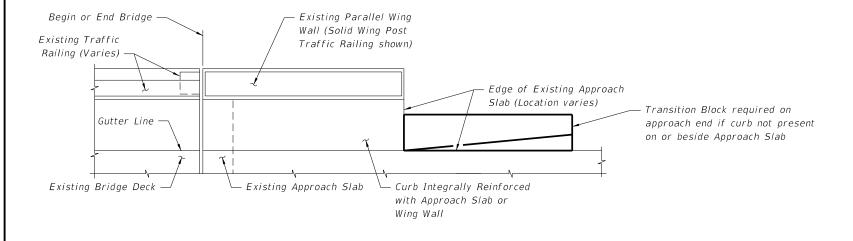
FDOT DESIGN STANDARDS 2013

GUARDRAIL TRANSITIONS AND CONNECTIONS FOR EXISTING BRIDGES *INDEX* SHEET NO. NO. 402 24



Existing Flared Wing Wall (Post & Rail Traffic Railing shown, Solid Wing Post similar). Parallel portion Begin or End Bridge of Wing Wall may or may not exist Existing Traffic Railing (Varies) Edge of Existing Approach Slab (Location varies) -Gutter Line Parallel Portion Flared Portion ı if Present – Existing Approach Slab Existing Bridge Deck -Curb Integrally Reinforced - Transition Block required on with Approach Slab or approach end if curb not present Wing Wall on or beside Approach Slab

PARTIAL PLAN VIEW OF EXISTING BRIDGE WITH FLARED WING WALLS AND PARALLEL INTEGRALLY REINFORCED APPROACH SLAB CURBS (APPROACH SLAB WITH DETACHED CURBS OR SIDEWALK SIMILAR)



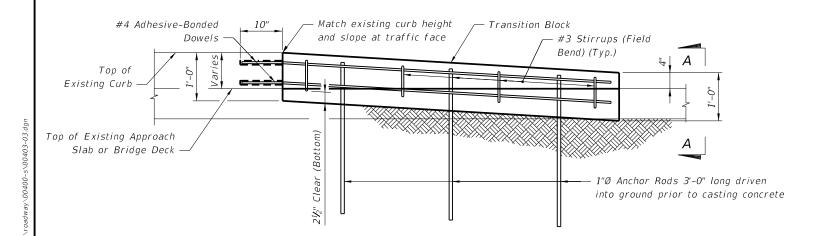
CROSS REFERENCE: For Transition Block Details. Quantities and reinforcement see Sheet 3.

PARTIAL PLAN VIEW OF EXISTING BRIDGE WITH PARALLEL WING WALLS AND INTEGRALLY REINFORCED APPROACH SLAB CURBS (APPROACH SLAB WITH DETACHED CURBS OR SIDEWALK SIMILAR)

LAST REVISION 07/01/05

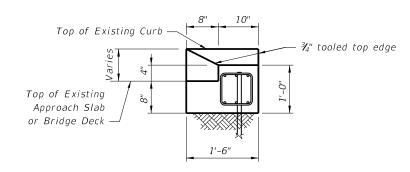
DESCRIPTION:



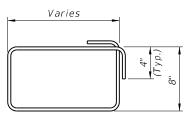


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUA	ANTITIE	TIES			
ITEM	UNIT	QUANTITY			
Concrete Class NS	CY	0.4			
Reinforcing Steel	LB	61			
Guardrail (Reset)	LF	12.5			



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

CONCRETE: Concrete for Transition Blocks shall be Class NS.

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

W BEAM GUARDRAIL: Guardrail components and installation shall be in accordance with Design Standards Index 400.

ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 (Type HV) and be installed in accordance with Specification Section 416.

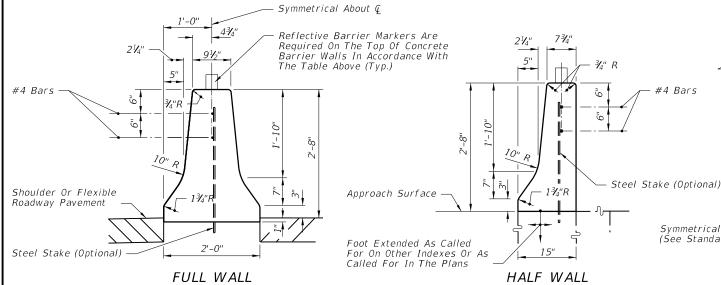
Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

PAYMENT: Payment for Guardrail work will be made under Pay Item Guardrail (Reset) LF. Payment for Transition Block will be made under Pay Item Concrete Curb (Special), LF.

LAST REVISION 01/01/10

DESCRIPTION:

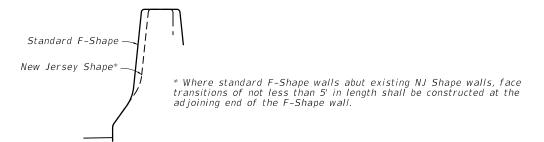




For concrete barrier wall details at piers, highway lighting and guardrail connections, see other sheets of this Index.

Standard barrier to be paid for under the contract unit price for Median Concrete Barrier Wall, LF.

STANDARD BARRIER WALL SECTIONS



WALL FACE SAFETY SHAPES

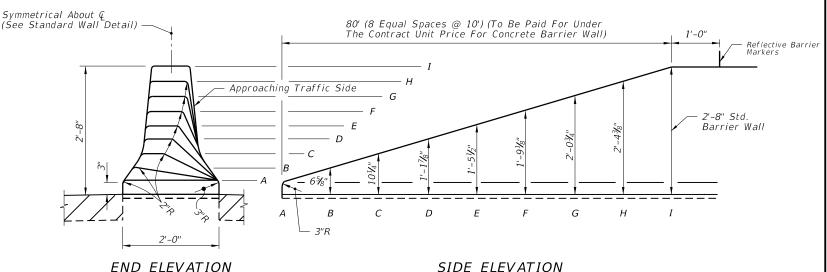
25' (5 Equal Spaces @ 5') (To Be Paid For Under The Contract Unit Price For Concrete Barrier Wall) 1'-0" Symmetrical About @ Reflective Barrier (See Standard Wall Detail) Approaching Traffic Side

> END ELEVATION SIDE ELEVATION

TO BE USED ONLY WHERE TERMINAL LOCATED CLEAR ZONE WIDTH FROM EDGE OF THE NEAR APPROACH TRAFFIC LANE.

CONCRETE BARRIER WALL TERMINAL

DETAIL II



DESIGN SPEED 45 MPH OR LESS CONCRETE BARRIER WALL TERMINAL FOR NARROW MEDIAN DETAIL III

GENERAL NOTES

- 1. Class II concrete shall be used for all reinforced and plain (nonreinforced) concrete barrier walls; except, in moderately and extremely aggressive environments, Class IV concrete shall be used. All reinforcing steel with undesignated size shall be #4 bars. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specifications, unless other finish called for in plans. The surfaces shall have a Class 5 Applied Finished Coating in accordance with Section 400 only when called for in the plans.
- Concrete barrier wall terminal notes for design speeds ≥ 50 mph
 - a. Terminated outside clear zone of the approach traffic with 'DETAIL II' end treatment.

DESCRIPTION:

- b. Terminated within a shielded location.c. Terminal protection by the use of a crash cushion system.d. Terminated in conjunction with a suitably designed transition to another barrier.
- 3. Expansion joints in wall required only at bridge ends and/or at locations where wall is an integral part of existing or proposed concrete slab; wall joints are to match an existing or proposed expansion joint.

- 4. When the barrier is installed adjacent to the pavement the top 12" of the subgrade shall be compacted to at least 98% of the maximum density determined by FM 1-T 180, Method D.
- 5. For cast-in-place barrier wall segments constructed with the slip form method, score 3/8" deep crack control V-Grooves while the concrete is still plastic and mold them when walls are constructed with the stationary form method. All 3/8" deep V-Grooves shall be spaced at 20' intervals, the end of the side face grooves shall be in line with the ends of the top face groove and the long dimension of all grooves shall align at 90 degrees to the longitudinal axis of the wall. When wall segments are less than 40' in length, space the V-Groove equally between open joints. Dowel transverse construction joints for abutting segments less than 40' (See Detail B).
- 6. Precast construction is allowed as an alternate to cast-in-place construction.
- a. Wall segments <40' in length shall be joined by a transverse joint in accordance with Details C & D on Sheet 2. The minimum segment length is 20'.
- b. Bedding of the precast sections shall be facilitated by the use of sand-cement grout or equal method to assure uniform bearing.
- c. Reinforcement may be required for handling stresses.

- On roadways designated for reverse laning all downstream, ends that are not shielded or outside the clear zone shall be marked by Type 3 Object
- 8. Cost of reinforcing steel and reflective barrier markers shall be included in the contract unit price for concrete barrier wall. See individual details for pay item information.
- 9. For barrier wall inlet details see Indexes Nos. 217, 218 and 219.
- 10. Concrete barrier wall with New Jersey Safety Shape may not be substituted for the Standard F Shape Barrier.

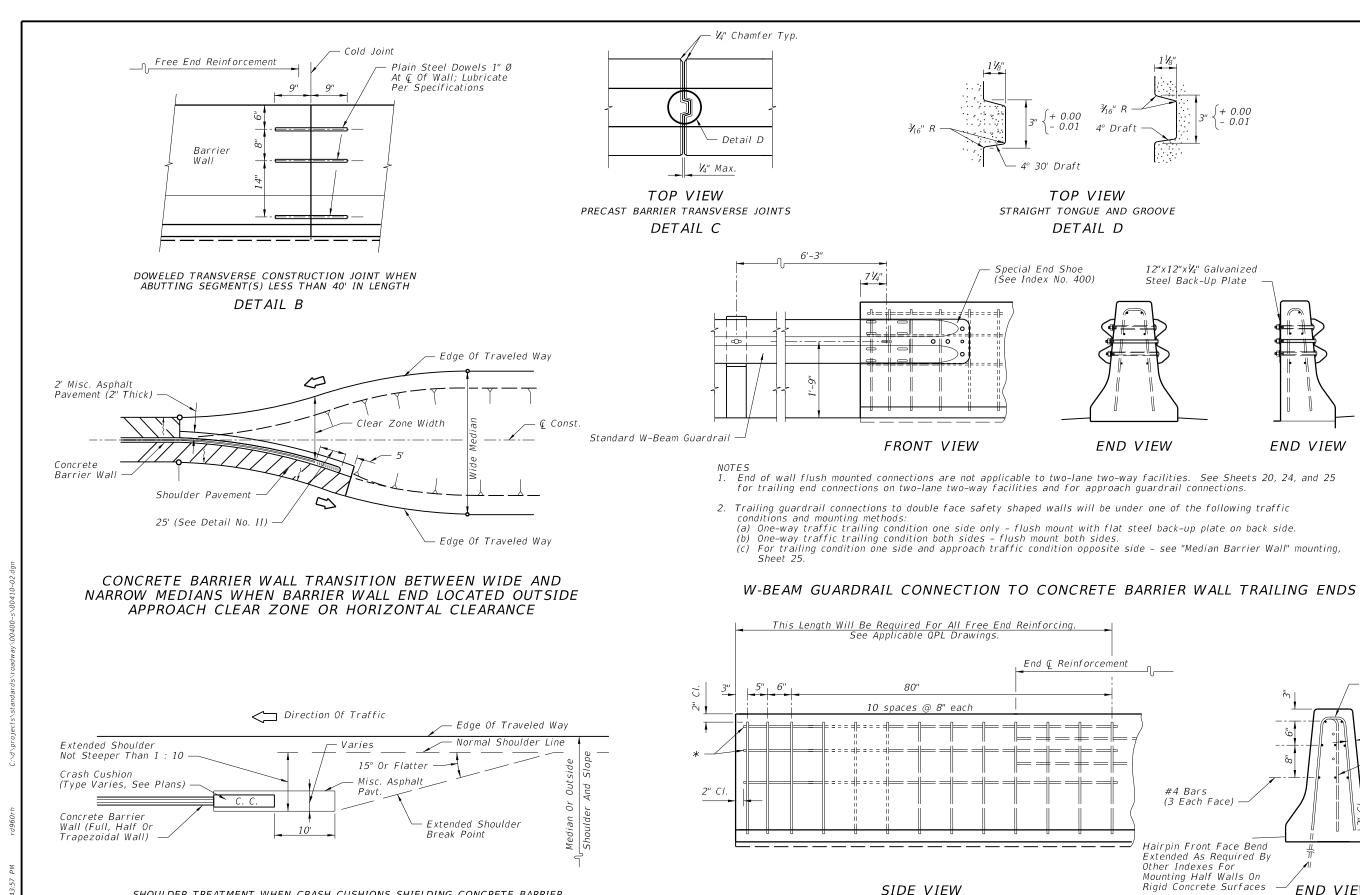
LAST REVISION 01/01/12



FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

INDEX SHEET NO. NO. 410



SHOULDER TREATMENT WHEN CRASH CUSHIONS SHIELDING CONCRETE BARRIER WALL END LOCATED INSIDE APPROACH CLEAR ZONE OR HORIZONTAL CLEARANCE

DETAIL A

Free end reinforcement required for nonreinforced walls at the following locations: All exposed ends; abutting ends of true joints; ends with guardrail connections; ends with redirective crash cushion connections; and, ends connecting to bridge traffic rails or other rigid barrier walls.

FREE END REINFORCEMENT

#4 Bars (3 Each Face)

12"x12"x1/4" Galvanized

END VIEW

Steel Back-Up Plate

DESCRIPTION: REVISION 07/01/05



FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

INDEXSHEET NO. NO.

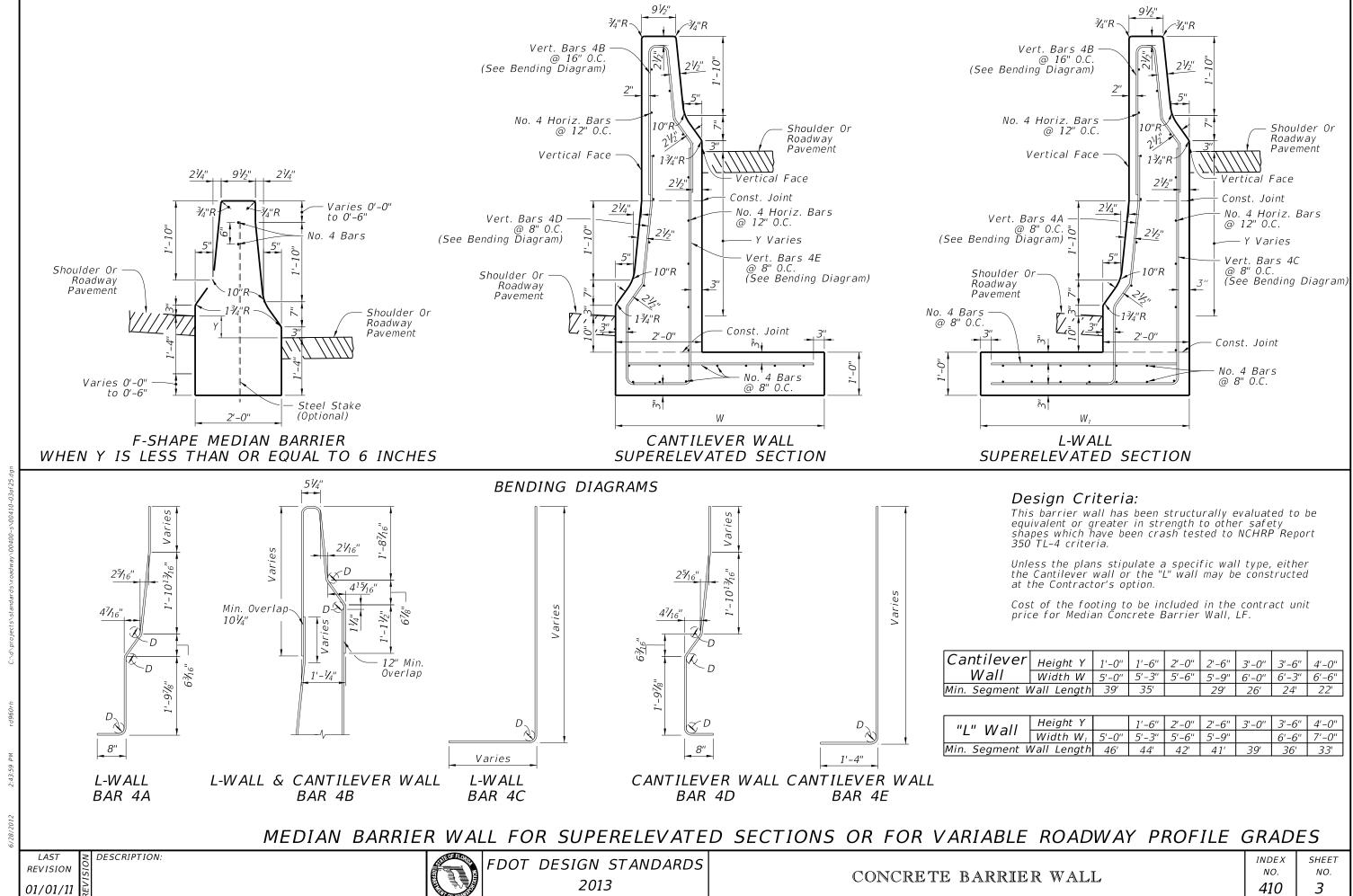
410

#4 Hairpins

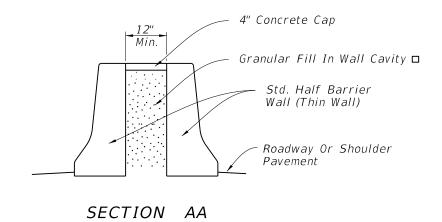
Half Wall

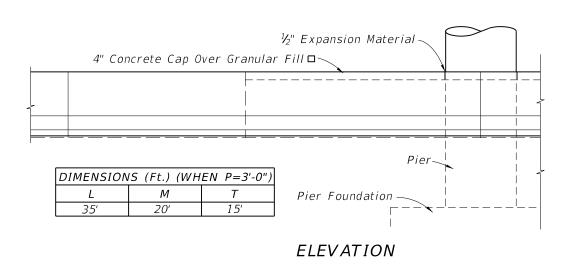
Hairpin

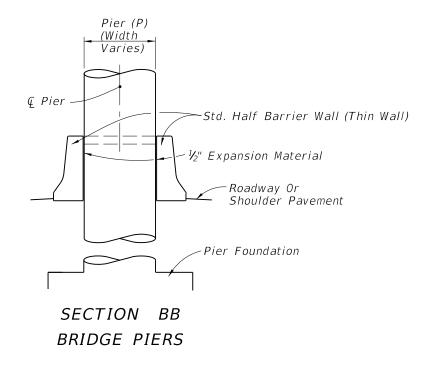
END VIEW











□ Fill To Be Free Of Deleterious And Cementitious Material

CONCRETE MEDIAN BARRIER WALL TRANSITIONS AT BRIDGE PIERS WHEN DESIGN SPEED < 45 MPH

LAST REVISION 07/01/07

DESCRIPTION:

FDOT DESIGN STANDARDS 2013

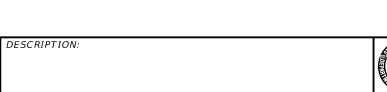
CONCRETE BARRIER WALL

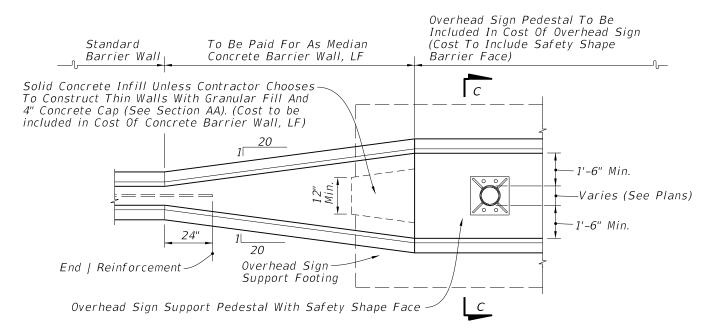
INDEX SHEET NO. NO.



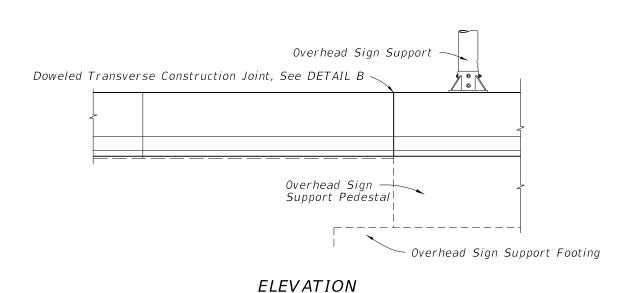


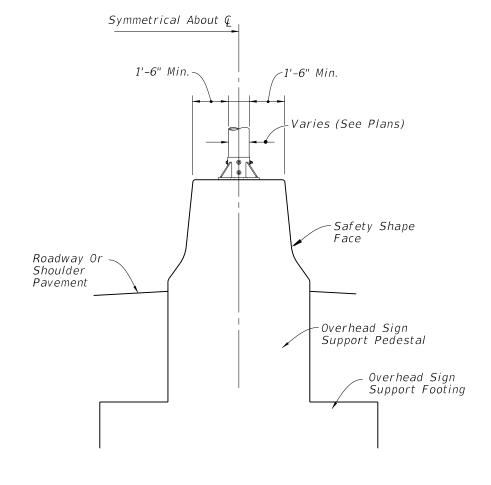






PLAN

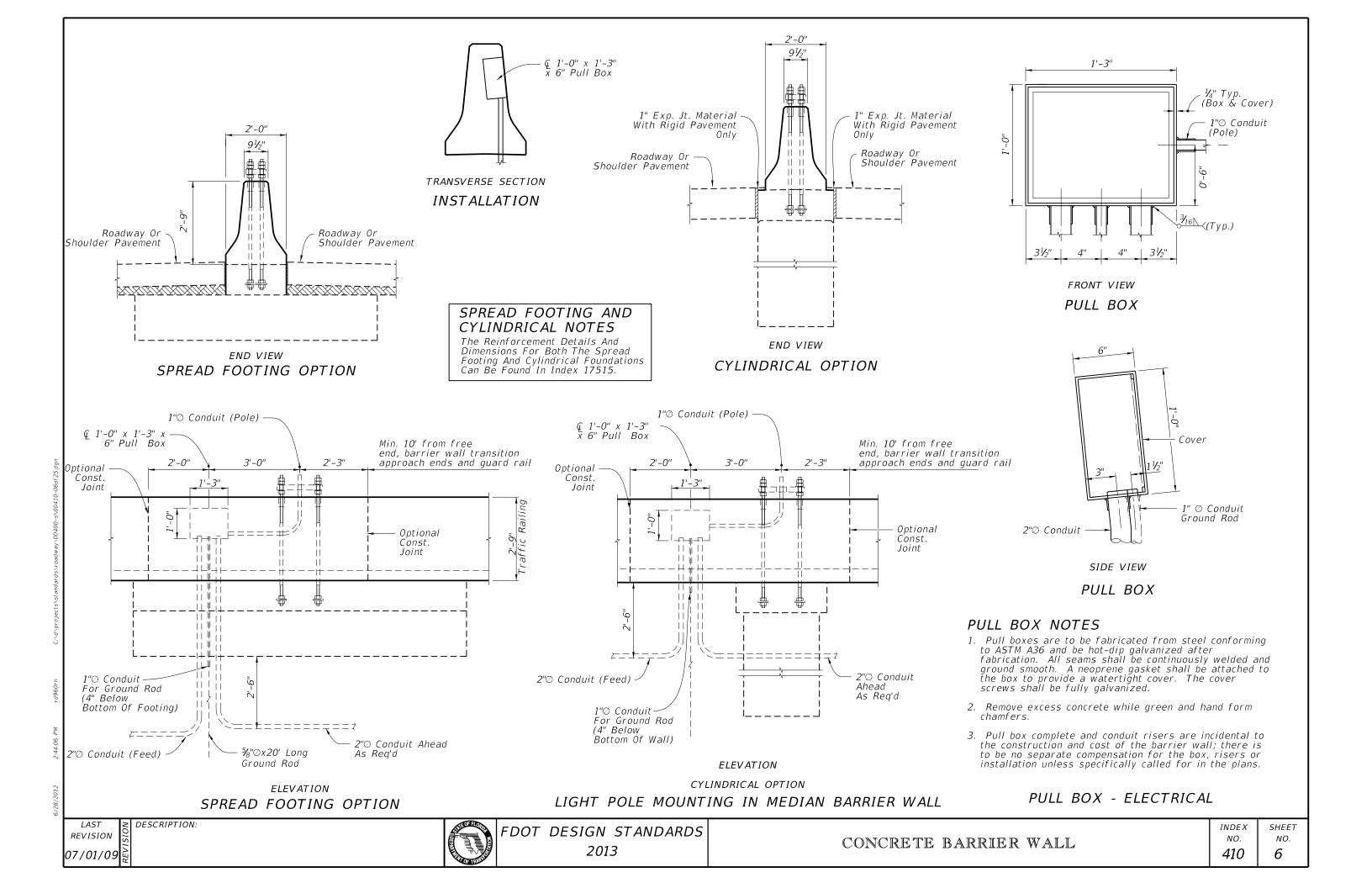


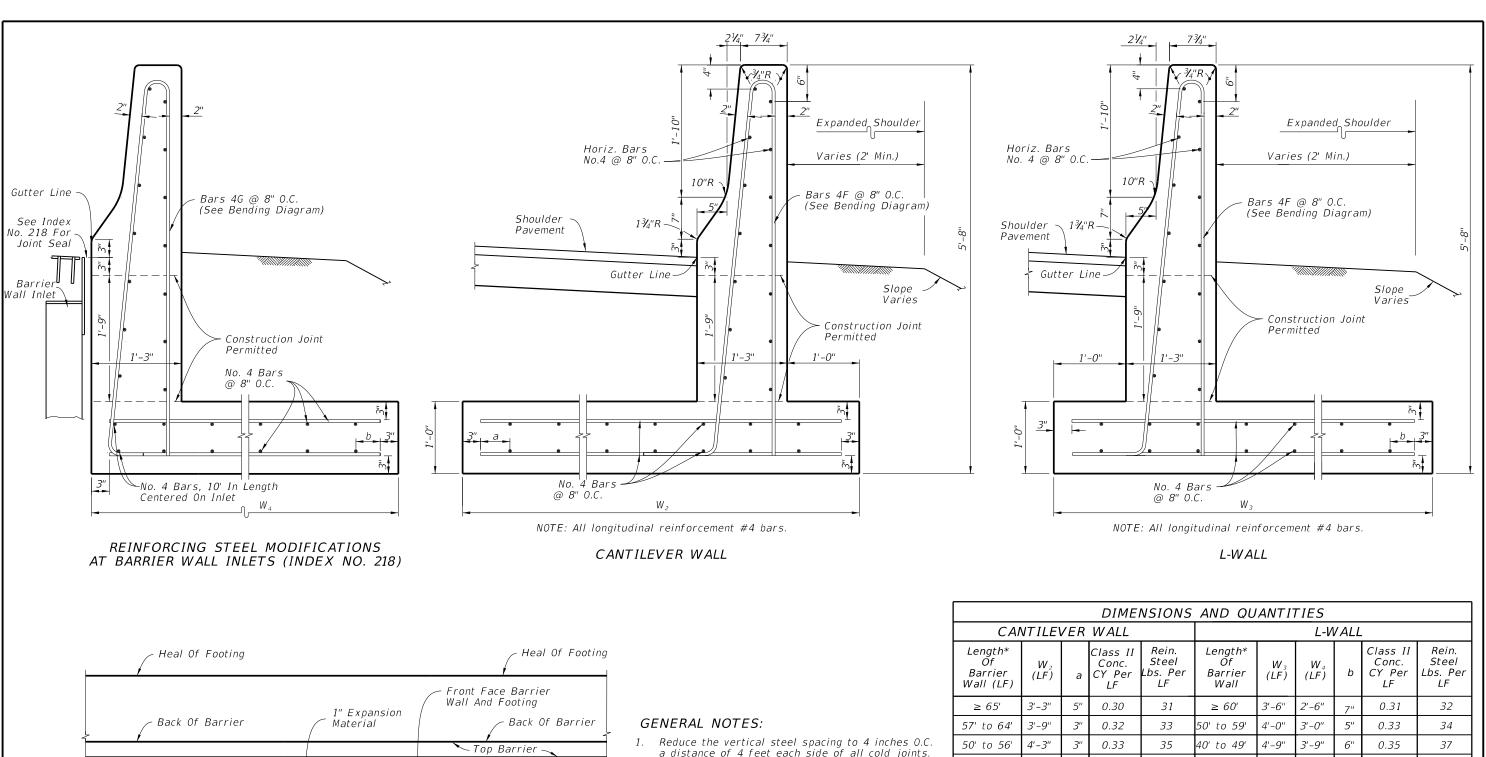


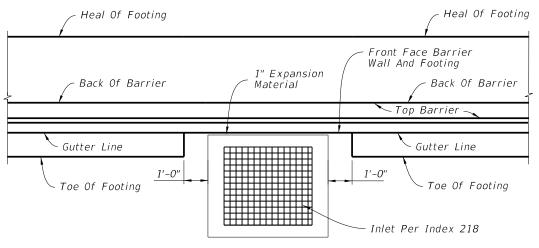
SECTION CC

CONCRETE MEDIAN BARRIER WALL TRANSITIONS AT OVERHEAD SIGN SUPPORTS WHEN DESIGN SPEED < 45 MPH

FDOT DESIGN STANDARDS 2013







BARRIER WALL FOOTING TRANSITION AT INLETS PLAN VIEW

- a distance of 4 feet each side of all cold joints.
- This barrier wall has been structurally evaluated to be equivalent or greater in strength to other safety shapes which have been crash tested to NCHRP Report 350 TL-4 criteria.
- 3. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder), LF.

DIMENSIONS AND QUANTITIES										
CANTILEVER WALL				L-W ALL						
Length* Of Barrier Wall (LF)	W ₂ (LF)	a	Class II Conc. CY Per LF	Rein. Steel Lbs. Per LF	Length* Of Barrier Wall	W₃ (LF)	W₄ (LF)	b	Class II Conc. CY Per LF	Rein. Steel Lbs. Per LF
≥ 65′	3'-3"	5"	0.30	31	≥ 60′	3'-6"	2'-6"	7"	0.31	32
57' to 64'	3'-9"	3"	0.32	33	50' to 59'	4'-0"	3'-0"	5"	0.33	34
50' to 56'	4'-3"	3"	0.33	35	40' to 49'	4'-9"	3'-9"	6"	0.35	37
41' to 49'	5'-0"	7"	0.36	38	35' to 39'	5'-3"	4'-3"	4"	0.37	39
36' to 40'	5'-6"	5"	0.38	40	30' to 34'	5'-9"	4'-9"	2"	0.39	42
28' to 35'	6'-6"	3"	0.42	44	25' to 29'	6'-6"	5'-6"	3"	0.42	44
25' to 27'	7'-0"	7"	0.44	46	20' to 24'	7'-6"	6'-6"	2"	0.45	49

Quantities shown are for information only. For method of payment see payment note. Barrier wall inlets (Index 218) shall be isolated from the barrier wall stem and footing by 1" expansion material.

* All walls may be made up of segments of 20' or more in length provided the segments are joined by a transverse joint in accordance with Detail B. Segments shall have dimensions same as walls above.

REINFORCED CONCRETE BARRIER WALL (SHOULDER)

DESCRIPTION: LAST REVISION 01/01/11

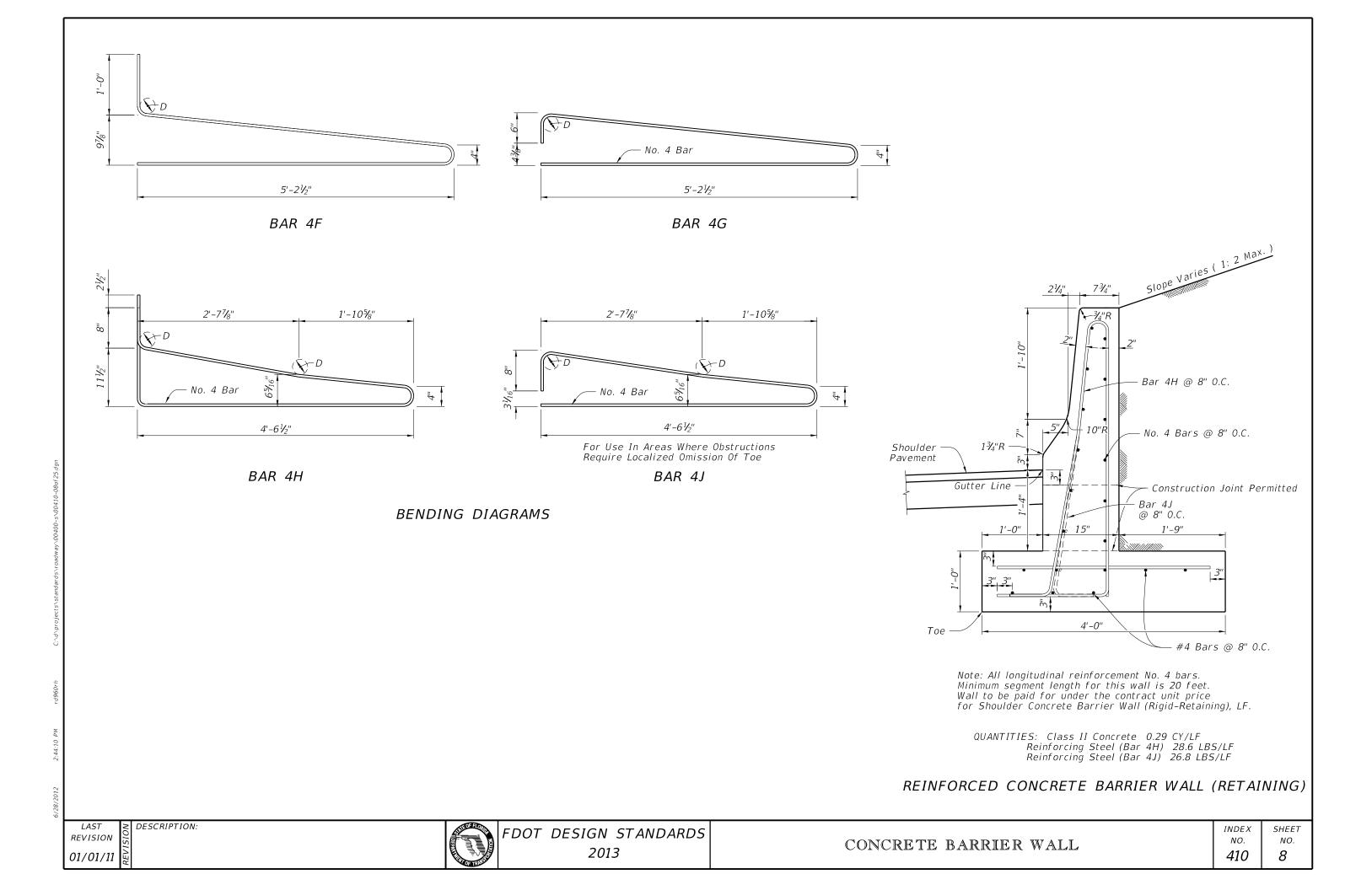


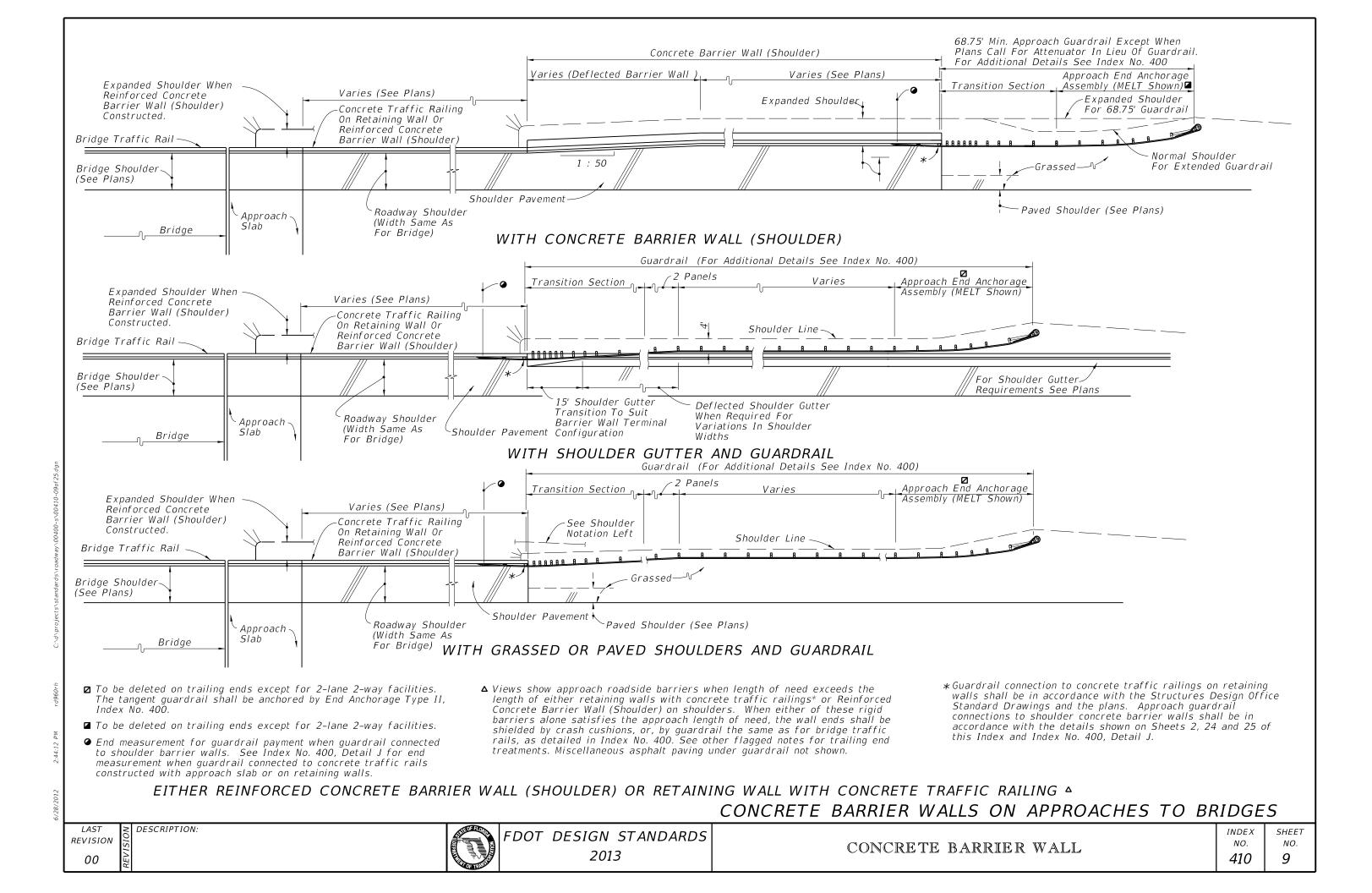
FDOT DESIGN STANDARDS 2013

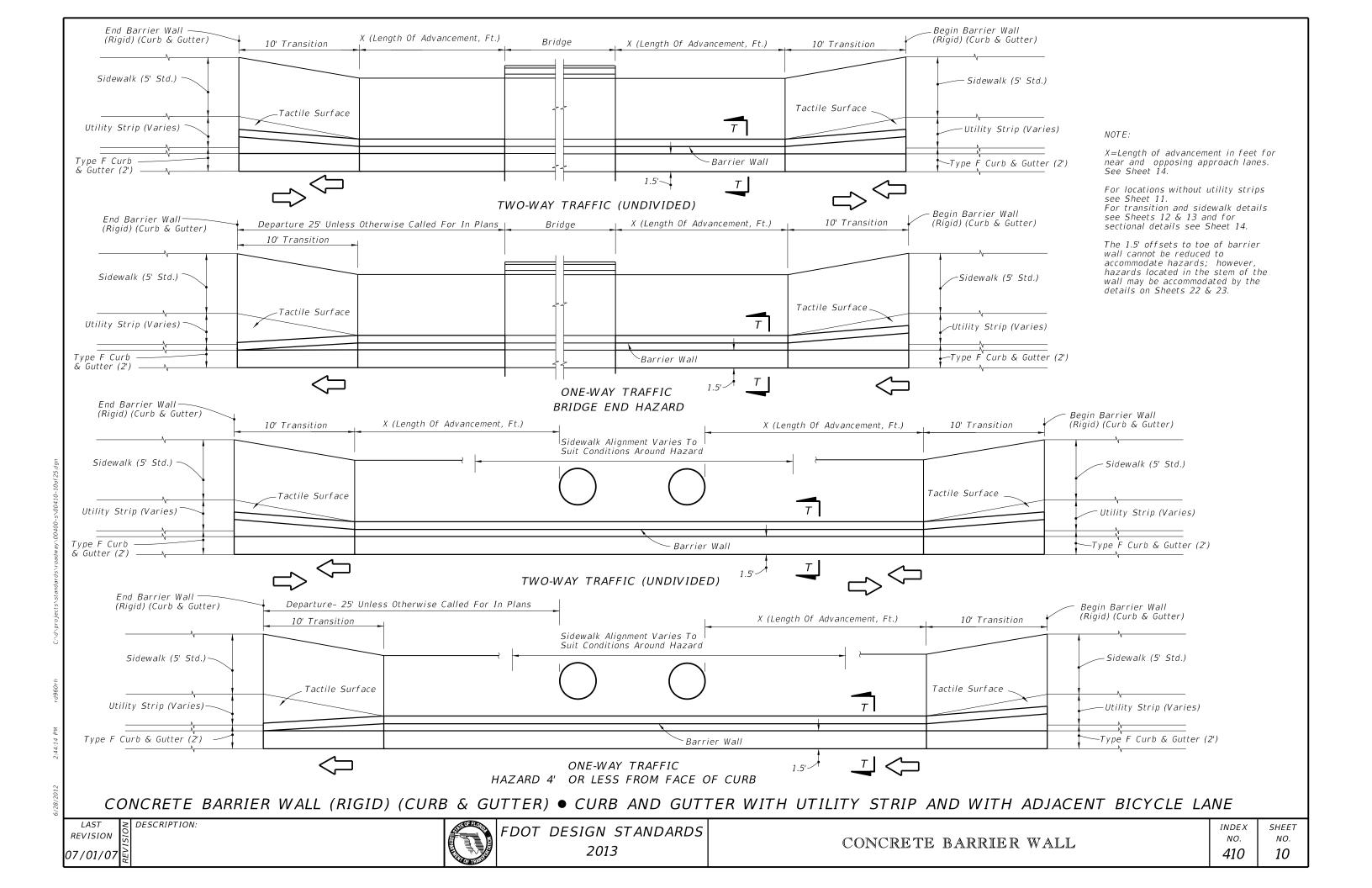
CONCRETE BARRIER WALL

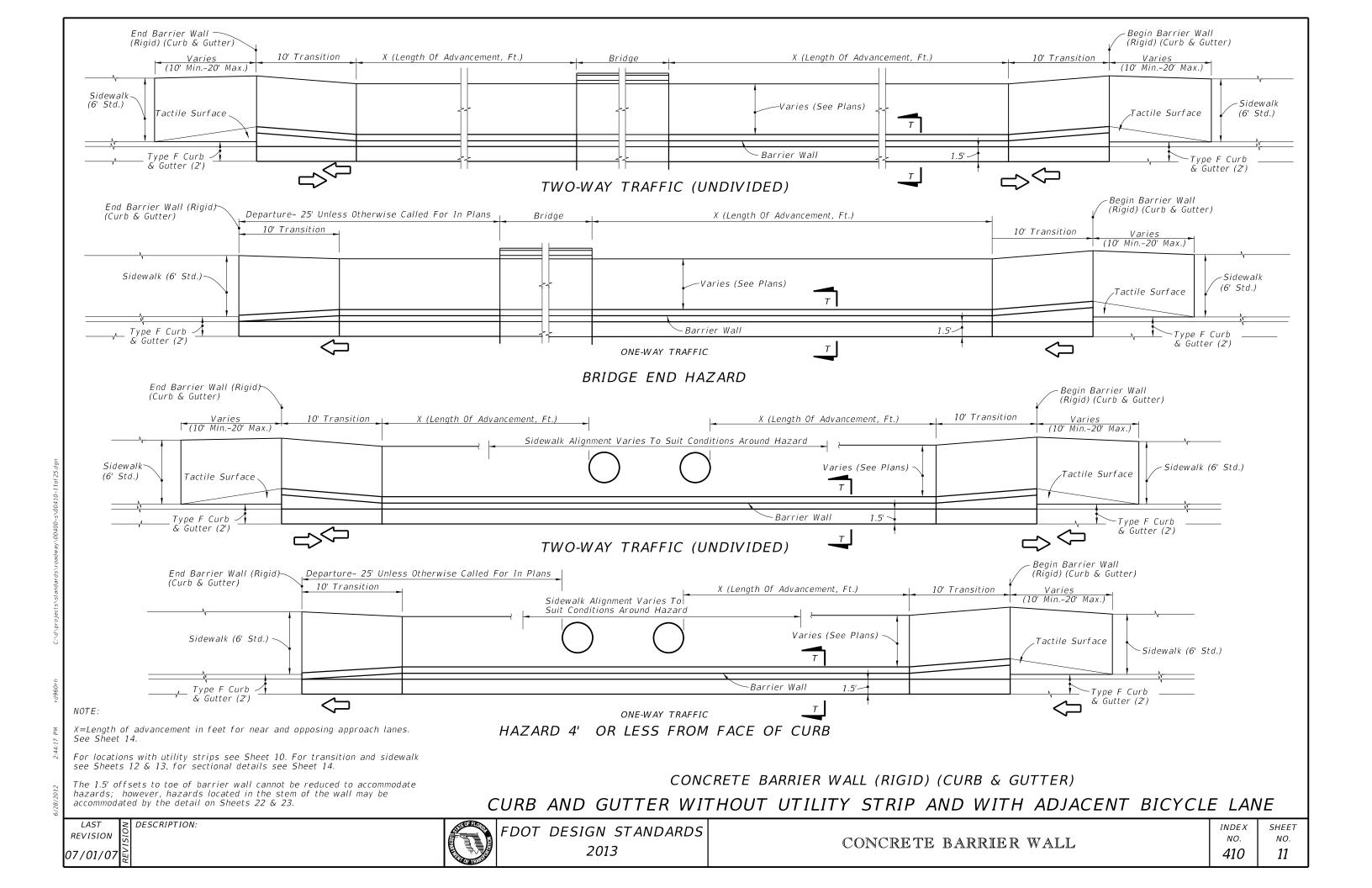
INDEX SHEET NO. 410

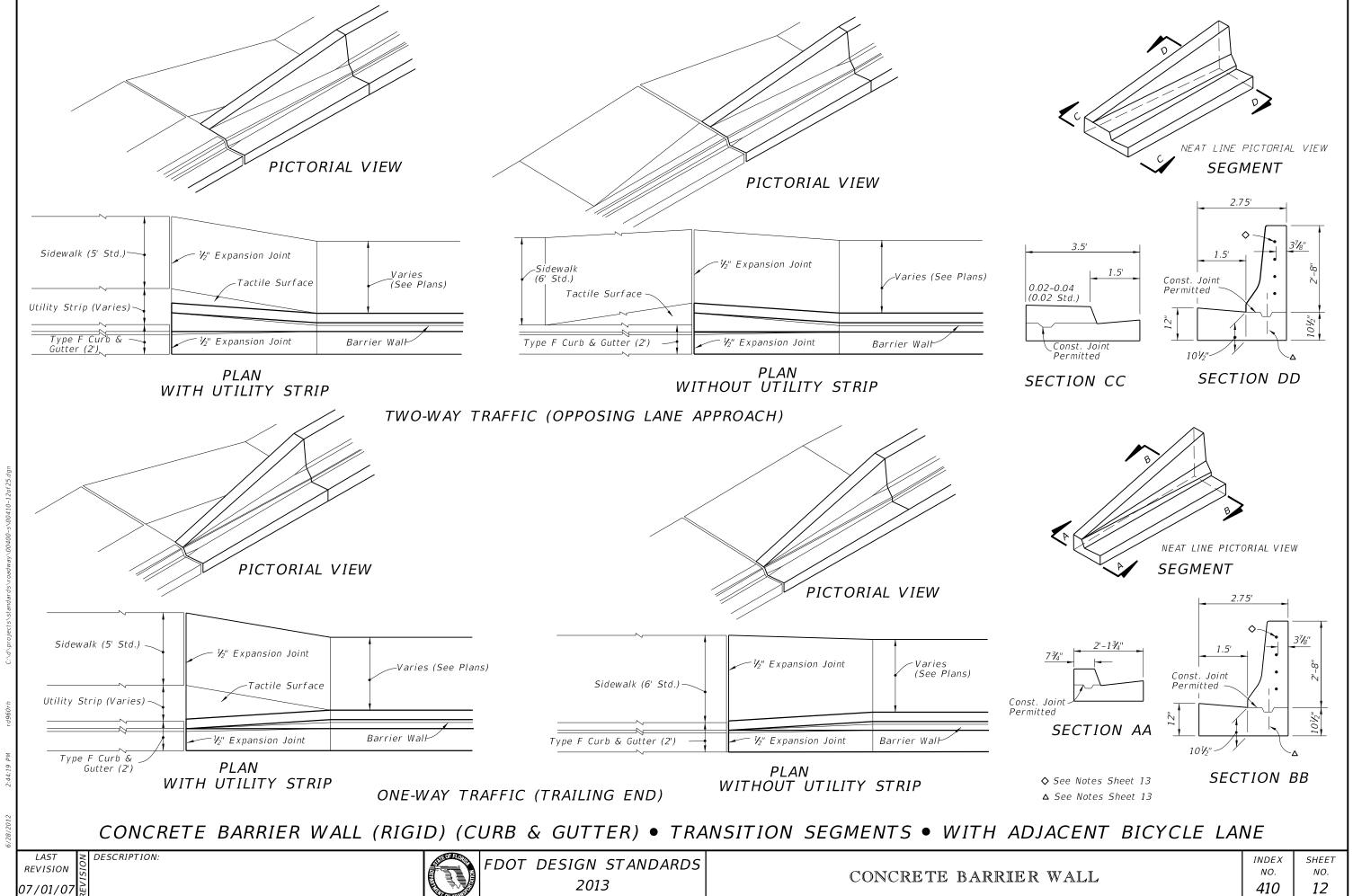
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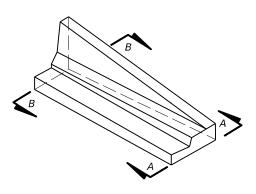








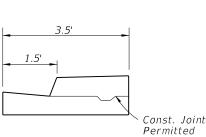


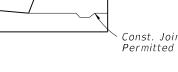


WITH OR WITHOUT UTILITY STRIP NEAT LINE PICTORIAL VIEW

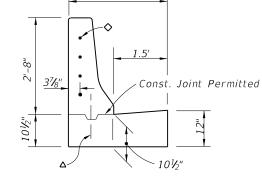
- ♦ Transition Segments Shall Be Doweled Into The End Of The Barrier Wall In The Following Manner: Four 1" diameter holes 6" deep on 6" centers shall be drilled in the end of the barrier and #6 bars 15" long set in an Adhesive Bonded Material System. The ends of the dowels extending into the transition segment shall be wrapped with one layer of 15 lb. Type I asphalt-saturated roofing felt with the ends crimped.
- △ When Construction Joints Are Utilized For Transition Segment Construction The Stem Shall Be Doweled To The Footing In The Following

Five #5 bars 15" long shall be embedded 7" into the footing. The dowels shall be spaced 15" on centers with the first dowel located 12" from the barrier wall. Dowels may be placed within or adjacent to the keyway.

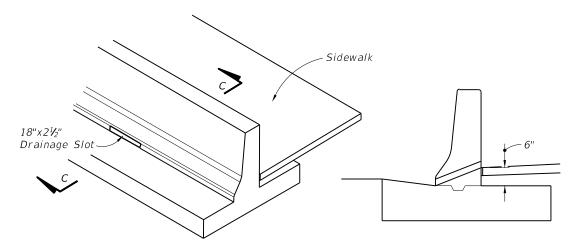




SECTION AA







NEAT LINE PICTORIAL VIEW

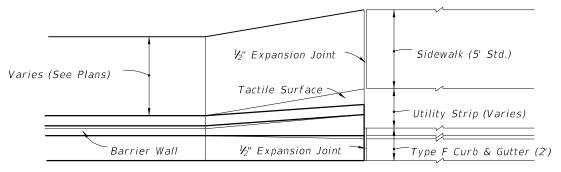
SECTION CC

NOTE: Drainage slots shall be located at all low points along the sidewalk, and, unless otherwise shown in the plans, slots shall be spaced at intervals not exceeding 50' in fill sections and 20' in cut sections. Slots shall be located such that only one bar is cut away or deleted in front and back lines of vertical reinforcement.

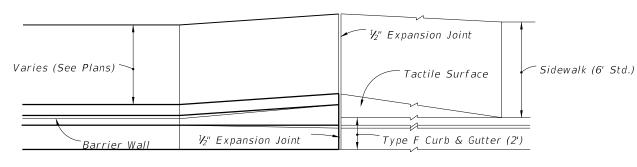
SIDEWALK DRAINAGE SLOT FOR BARRIER WALL (RIGID) (CURB & GUTTER)

PICTORIAL VIEW

PICTORIAL VIEW



PLANWITH UTILITY STRIP



PLAN WITHOUT UTILITY STRIP

RIGHT SIDE SHOWN, LEFT SIDE OPPOSITE HAND ONE-WAY AND TWO-WAY TRAFFIC (NEAR LANE APPROACH)

CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) ● TRANSITION SEGMENT ● WITH ADJACENT BICYCLE LANE

LAST REVISION



FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

SHEET INDEXNO. NO. 410 13

00

DESCRIPTION:

X (Length Of Advancement, Ft.)

OPPOSING LANE APPROACH

Tactile Surface

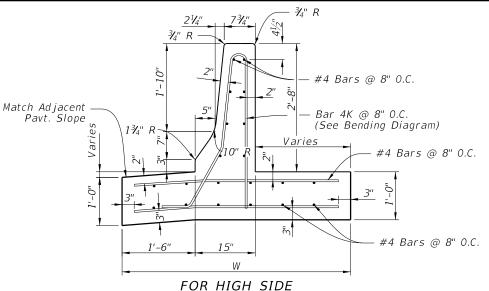
WITH OR WITHOUT UTILITY STRIP - UTILITY STRIP SHOWN SEE SHEET 10 & 11 FOR APPLICATIONS

Equation Variables:

esign peed mph	Length Of Advancement, Ft. (X)	
≤ 45	= 16 (D-d)	
Notor	The minimum length of advancement for	

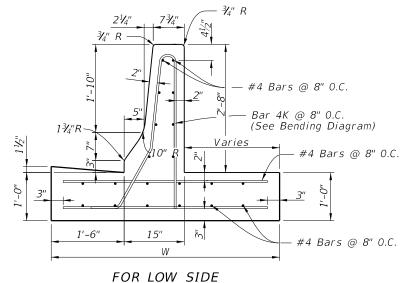
- D = Distance in feet from near edge of the near approach traffic lane to back of hazard or clear zone width whichever is lesser. For left side hazards and clear zones on two-way undivided facilities D is measured from the inside edge of the near approach traffic lane.
- d = Distance in feet from near edge of the near approach traffic lane to the face of barrier (at offset control point). For left side hazards on two-way undivided facilities d is measured from the inside edge of the nearest opposing traffic lane.

LENGTH OF ADVANCEMENT



SECTION	TT
QUANTIT	IES

QUANTITIES					
Length* Of Barrier Wall (LF)	W LF	Class II Conc. CY Per LF	Rein. Steel Lbs Per LF		
>73'	4'-9"	0.26	29		
56' to 73'	5'-6"	0.29	32		
48' to 55'	6'-0"	0.31	34		
41' to 47'	6'-6"	0.33	37		
36' to 41'	7'-0"	0.35	39		
29' to 35'	8'-0"	0.38	42		



SECTION TT

Note: All longitudinal reinforcement No. 4 bars. Shorter segments due to construction or expansion joint shall be doweled in the manner described for 'Transition Segments' on Sheet 13.

Transverse expansion joints are to be constructed at the juncture of wall transitions and curb and gutter, and at intervals so that spacing will not exceed 100'.

For barrier wall inlet details see Index No. 219. Inlet extends into bicycle lane 12". Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Curb & Gutter), LF.

CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) WITH ADJACENT BICYCLE LANE

LAST REVISION 07/01/09

DESCRIPTION:



FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

INDEX NO. 410

SHEET NO. 14

Begin Concrete Barrier Wall

_Type F Curb & Gutter

Point Of Departure

End Of Bridge Rail

That Requires Shielding

81/4"

BAR 4K

BENDING DIAGRAM

Or Other Hazard

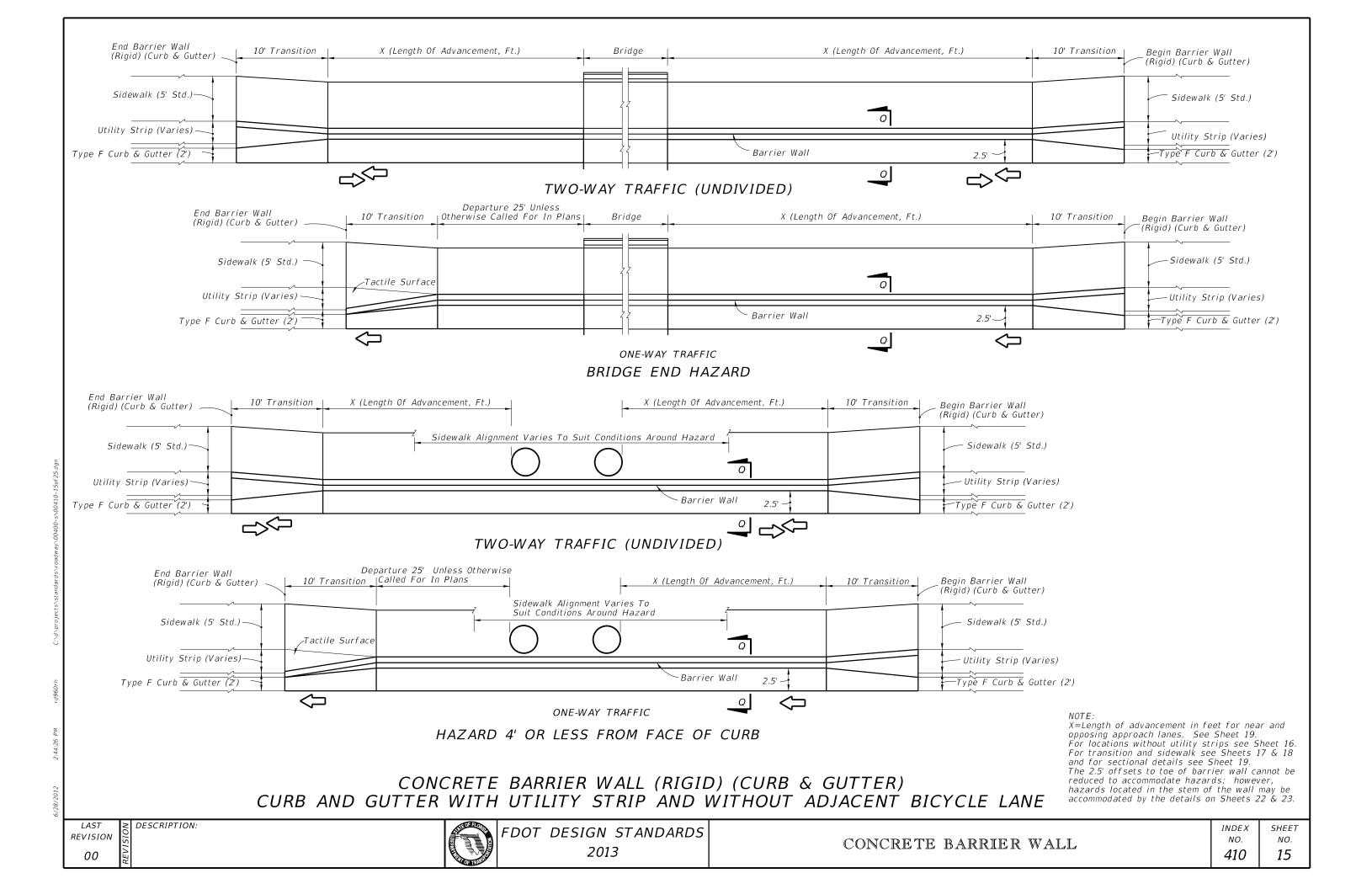
Departure Line

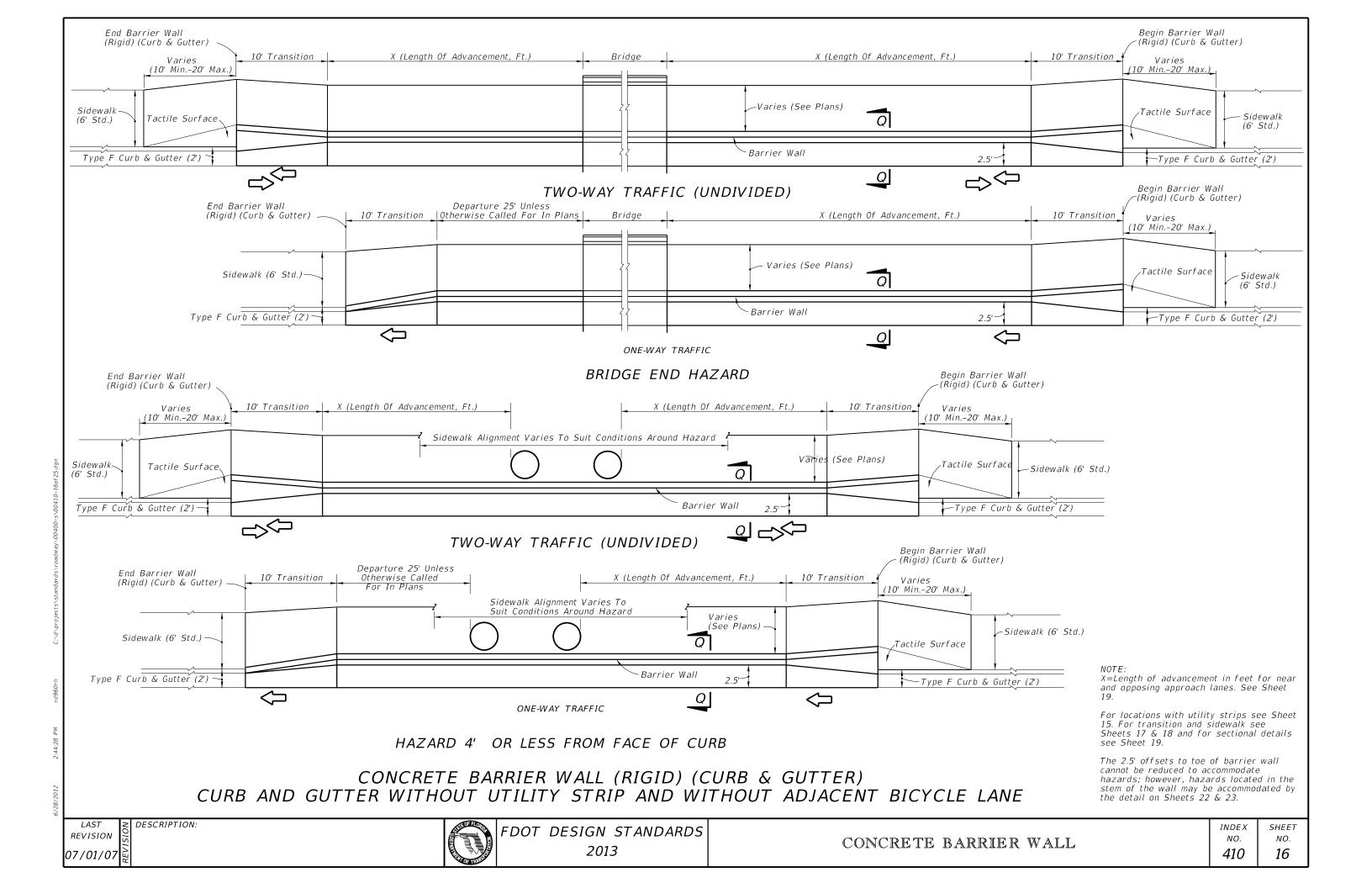
(Rigid) (Curb & Gutter)

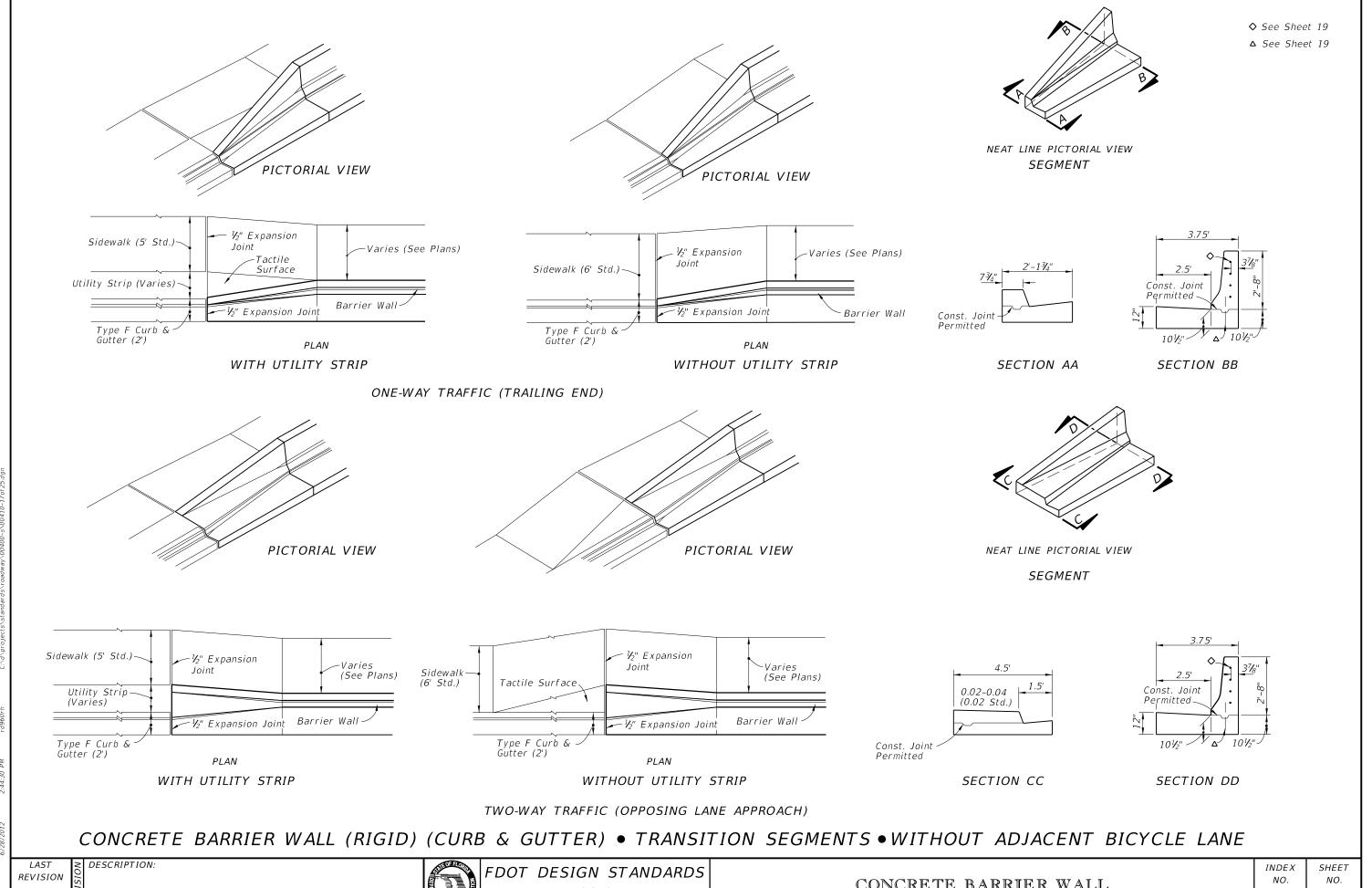
Note: The minimum length of advancement for both near and opposing lane approaches is 40'.

End Of Bridge Rail Or Other Hazard That Requires_

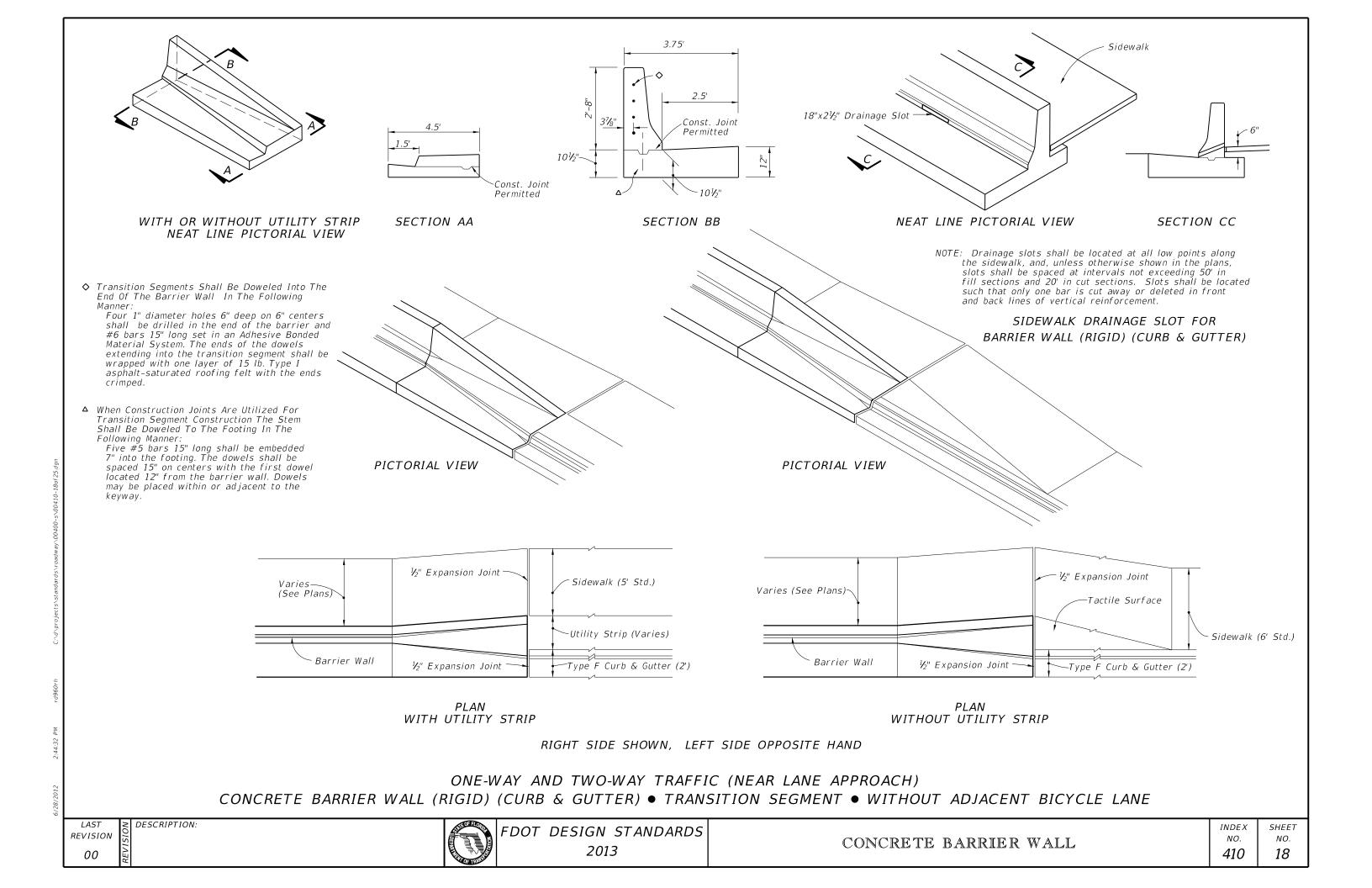
Shielding







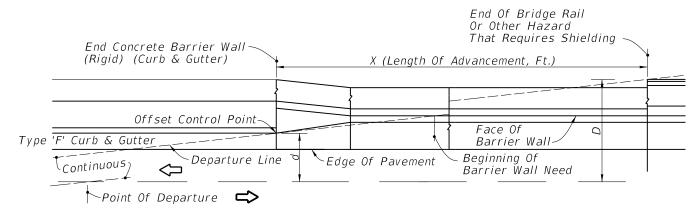
07/01/07



End Of Bridge Rail

NEAR LANE APPROACH

RIGHT SIDE APPROACH SHOWN - LEFT SIDE OPPOSITE HAND



OPPOSING LANE APPROACH

WITH OR WITHOUT UTILITY STRIP - UTILITY STRIP SHOWN SEE SHEET 15 & 16 FOR APPLICATIONS

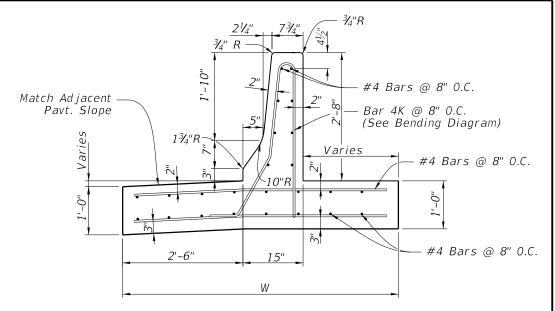
Equation Variables:

	Design Speed mph	Length Of Advancement, Ft.(X)
	≤ 45	16 (D-d)
	Note: T	The minimum length of advancement

for both near and opposing lane approaches is 40'.

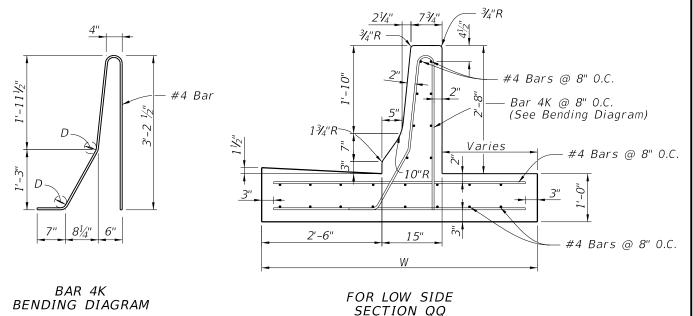
- D= Distance in feet from near edge of the near approach traffic lane to back of hazard or clear zone width whichever is lesser. For left side hazards and clear zones on two-way undivided facilities D is measured from the inside edge of the near approach traffic lane.
- d= Distance in feet from near edge of the near approach traffic lane to the face of curb (at offset control point). For left side hazards on two-way undivided facilities d is measured from the inside edge of the nearest opposing traffic lane.

LENGTH OF ADVANCEMENT



QUANTITIES				
Length* Of Barrier Wall (LF)	W LF	Class II Conc. CY Per LF	Rein. Steel Lbs Per LF	
>73'	4'-9"	0.26	29	
56' to 73'	5'-6"	0.29	32	
48' to 55'	6'-0"	0.31	34	
41' to 47'	6'-6"	0.33	37	
36' to 41'	7'-0"	0.35	39	
29' to 35'	8'-0"	0.38	42	

FOR HIGH SIDE SECTION QQ



Note: All longitudinal reinforcement #4 bars. Shorter segments due to construction or expansion joint shall be doweled in the manner described for 'Transition Segments' on Sheet 18.

Transverse expansion joints are to be constructed at the juncture of wall transitions and curb and gutter, and at intervals so that spacing will not exceed 100'.

For barrier wall inlet details see Index No. 219. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Curb & Gutter), LF.

CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) • WITHOUT ADJACENT BICYCLE LANE

LAST REVISION 07/01/09

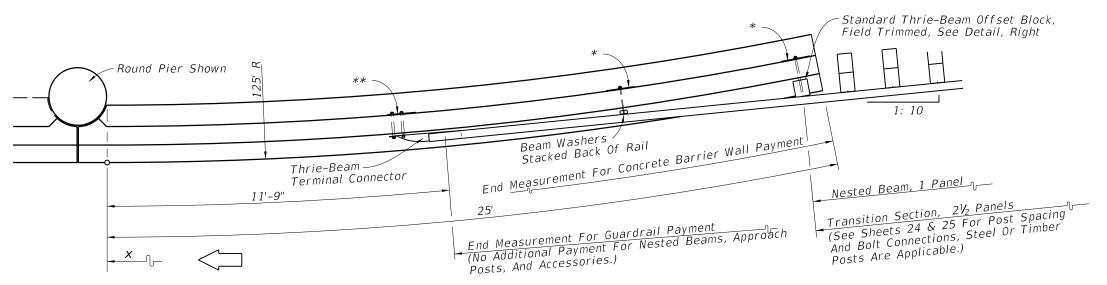
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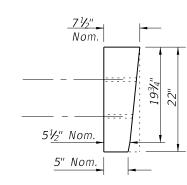


FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

SHEET *INDEX* NO. NO. 410 19



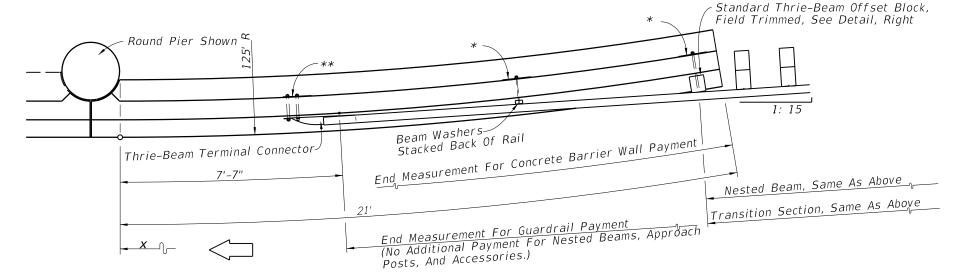


FOR USE WITH EITHER 1: 10 OR 1: 15 GUARDRAIL TRANSITIONS

STANDARD THRIE-BEAM OFFSET BLOCK (FIELD TRIMMED)

PLAN FOR DESIGN SPEED ≤ 45 MPH

For details at Rigid Hazard see Sheet 21.



PLAN FOR DESIGN SPEED ≥ 50 MPH

Note: For continuous barrier between independent bents or single pier columns see Sheets 21-23.

ARC LENGTH (FT)	DISTANCE "x" (FT)	OFFSETS "y" (FT)	125 F
4	4.00	0.06	
8	7.99	0.26	Y
12	11.98	0.58	X
16	15.96	1.02	
20	19.91	1.60	Note:
21	20.91	1.76	Wall may be constructed
24	23.85	2.30	in chords having lengths ≤ 4 feet.
25	24.83	2.49	<u> </u>

SHOULDER BARRIER WALL AT ABOVE GROUND RIGID HAZARDS WHEN GUARDRAIL OFFSET FROM HAZARD LESS THAN 3'

NOTES

- 1. This wall is intended for use where the wall has bearing against the hazard; when the length between bent supports or pier columns exceeds 13', the affected segments shall be constructed in accordance with the detail for 'Reinforced Concrete Barrier Wall (Shoulder)', 'Section TT' or 'Section QQ', this index. In cases where the barrier wall and slope pavement or other structure would occupy the same location, the wall and structure are to be modified as detailed in the
- 2. The barrier wall radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane two-way facilities. On trailing ends of two-way multilane and one-way facilities the end connection on Sheet 1 may be used. For walls with normal offsets from hazards and their guardrail connections, see Sheet 24 & 25.
- 3. Refer to Index No. 400 for additional guardrail information.
- 4. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder), LF.
- * $12"x12"x\frac{1}{4}"$ galvanized steel back-up plate with $\frac{4}{8}"$ post bolts (either 14" or 18" long) and nuts with $\frac{5}{8}$ " plain round washers under nuts.
- ** Attach thrie-beam terminal connector to shoulder barrier wall with a 21"x12"x¾" thrie beam terminal connector plate and $5-\frac{1}{6}$ "x12" long HS hex bolts and nuts with $\frac{1}{6}$ " plain round washers under heads and nuts.

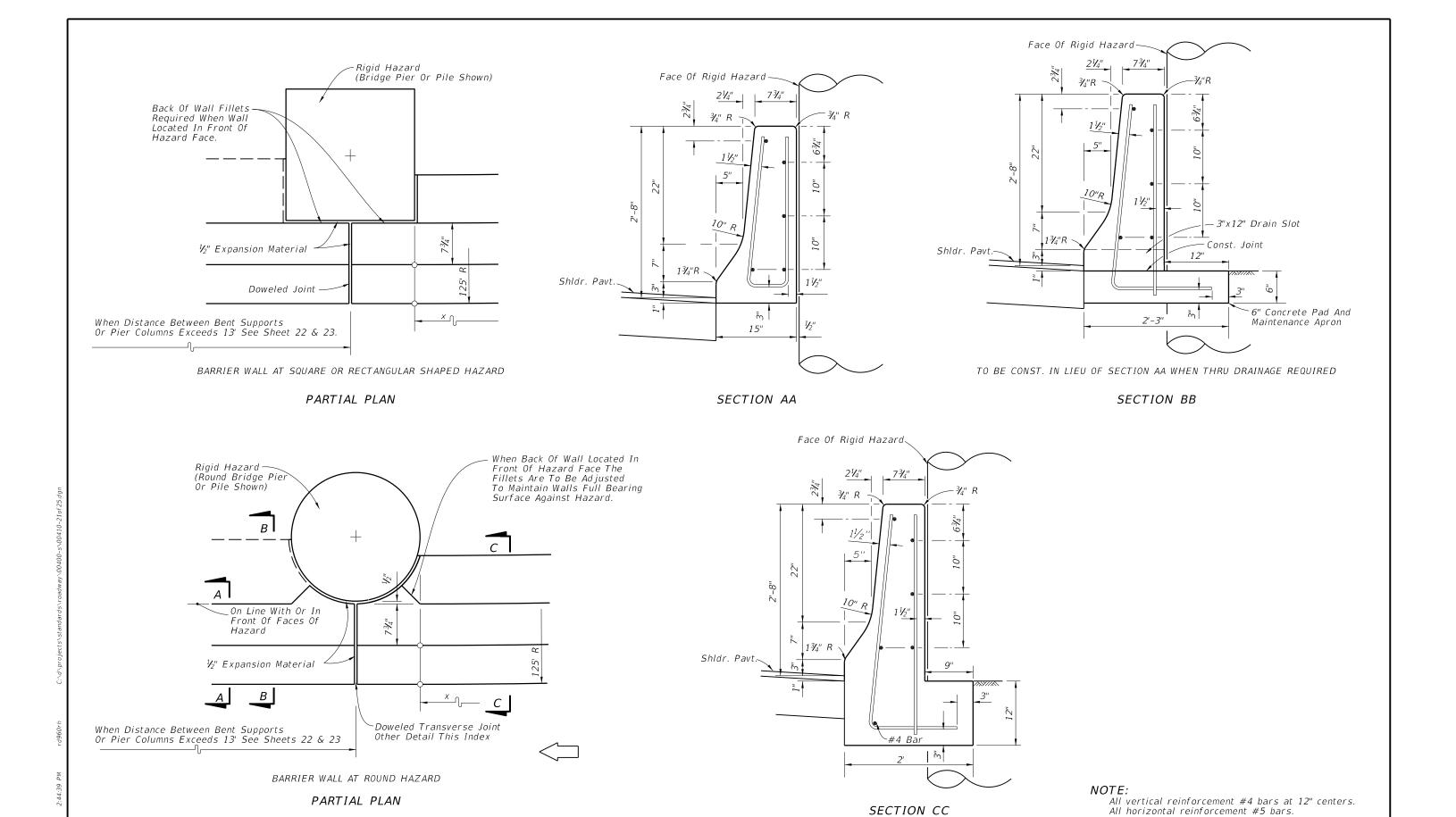
FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL

SHEET *INDEX* NO. NO.

20

DESCRIPTION:

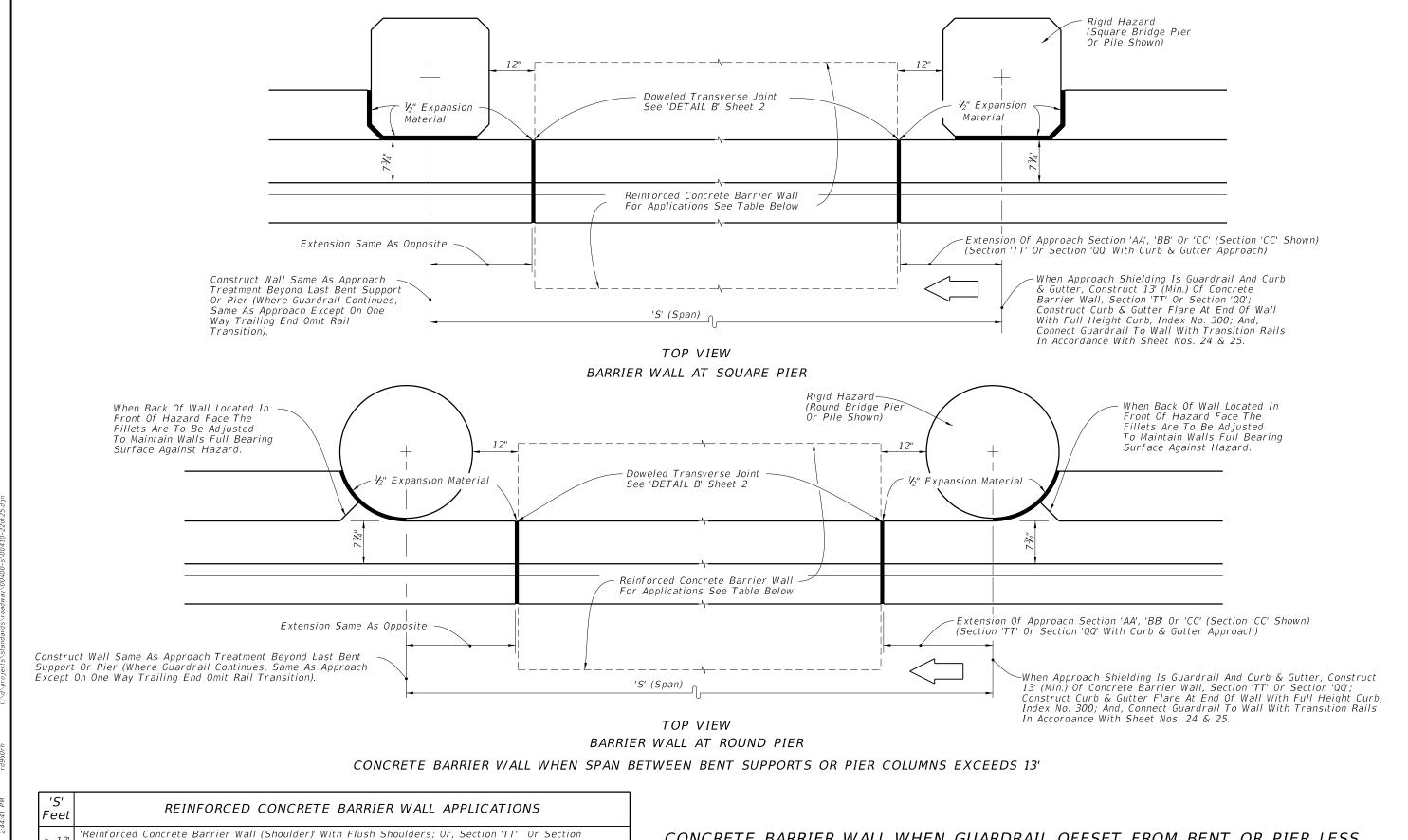


SHOULDER BARRIER WALL AT ABOVE GROUND RIGID HAZARDS WHEN GUARDRAIL OFFSET FROM HAZARD LESS THAN 3'

LAST REVISION S

DESCRIPTION:





Barrier wall footings that conflict with bent or pier foundations shall be modified as described in the plans.

CONCRETE BARRIER WALL WHEN GUARDRAIL OFFSET FROM BENT OR PIER LESS THAN 3 FEET OR WHERE WALL STEM ABUTS SUPPORTS OR PIER COLUMN

LAST REVISION 00

'QQ' With Curb & Gutter

DESCRIPTION:



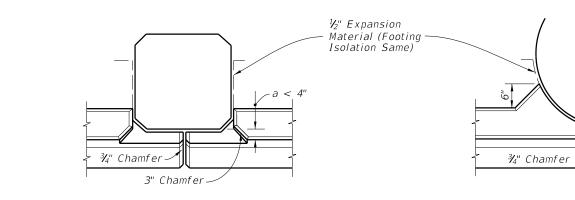
FDOT DESIGN STANDARDS 2013

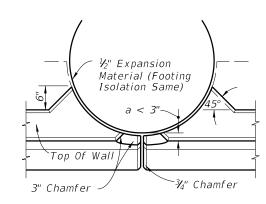
INDEX SHEET NO. 22

REVISION

07/01/07







TOP VIEWS

'a' Varies (Circular Or Octagonal Hazard Not More Than 2" In Front Of Face Of Wall). Applicable To Sections 'AA' And 'BB' With Spans Of ≤ 13', And To Section 'CC', Sheet No. 21. Applicable To Other Rigid Walls Of This Index For Spans > 13' Unless Otherwise Shown In The Plans.

HAZARD PENETRATING STEM OF RIGID CONCRETE BARRIER WALLS

The details on sheets 22 & 23 are treatments to the F-shape concrete barrier walls depicted on Sheet Nos. 9 through 19, where site conditions impose reduced clearances between above ground hazards and the walls. Bridge bent supports and piers are shown. These treatments are not applicable to hazards that cannot provide lateral support for the walls. See the plans for limits of wall sections applied and other associated wall treatments.

a ≥ 3"

Face Of Wall

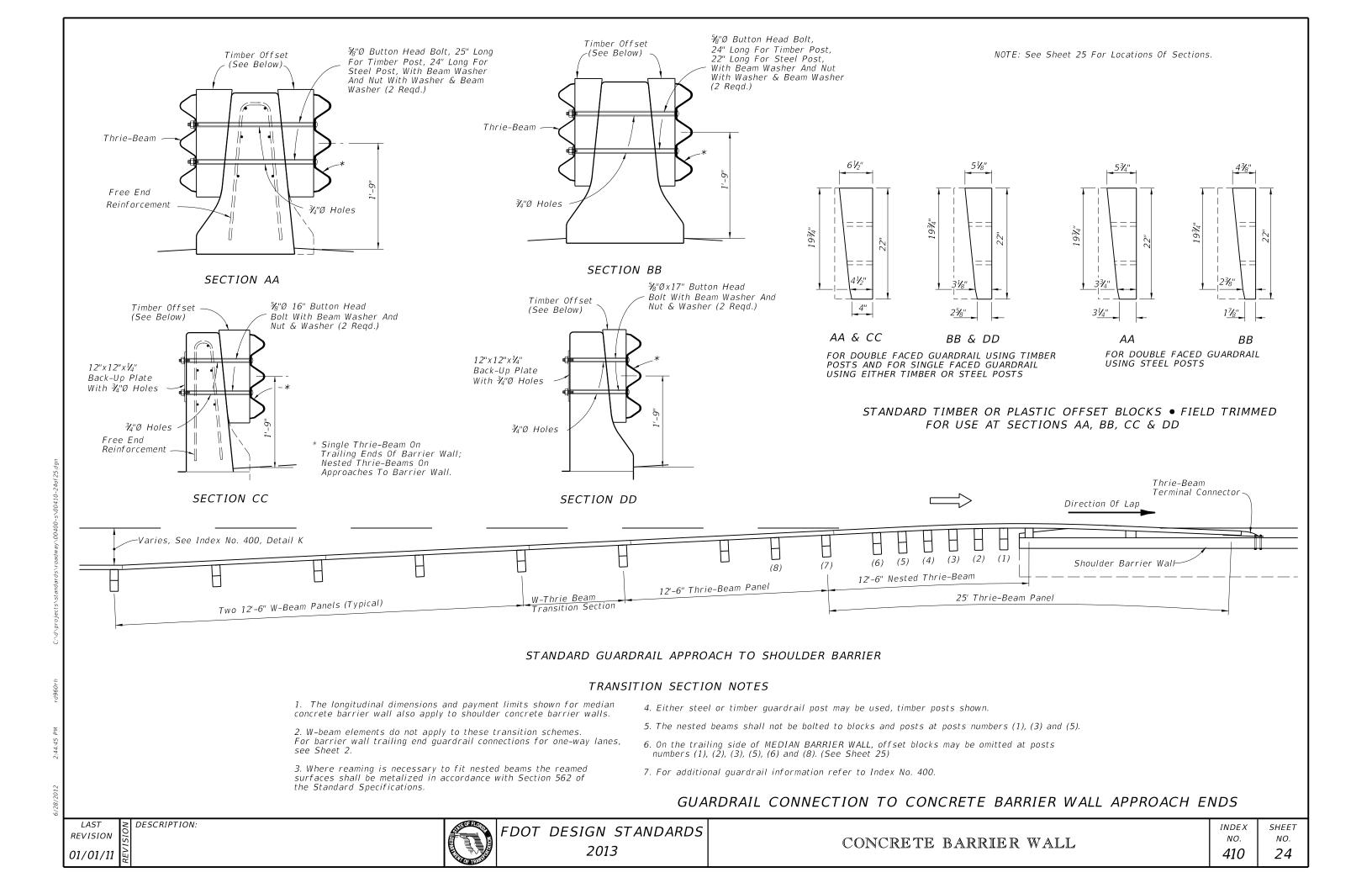
Toe Of Wall

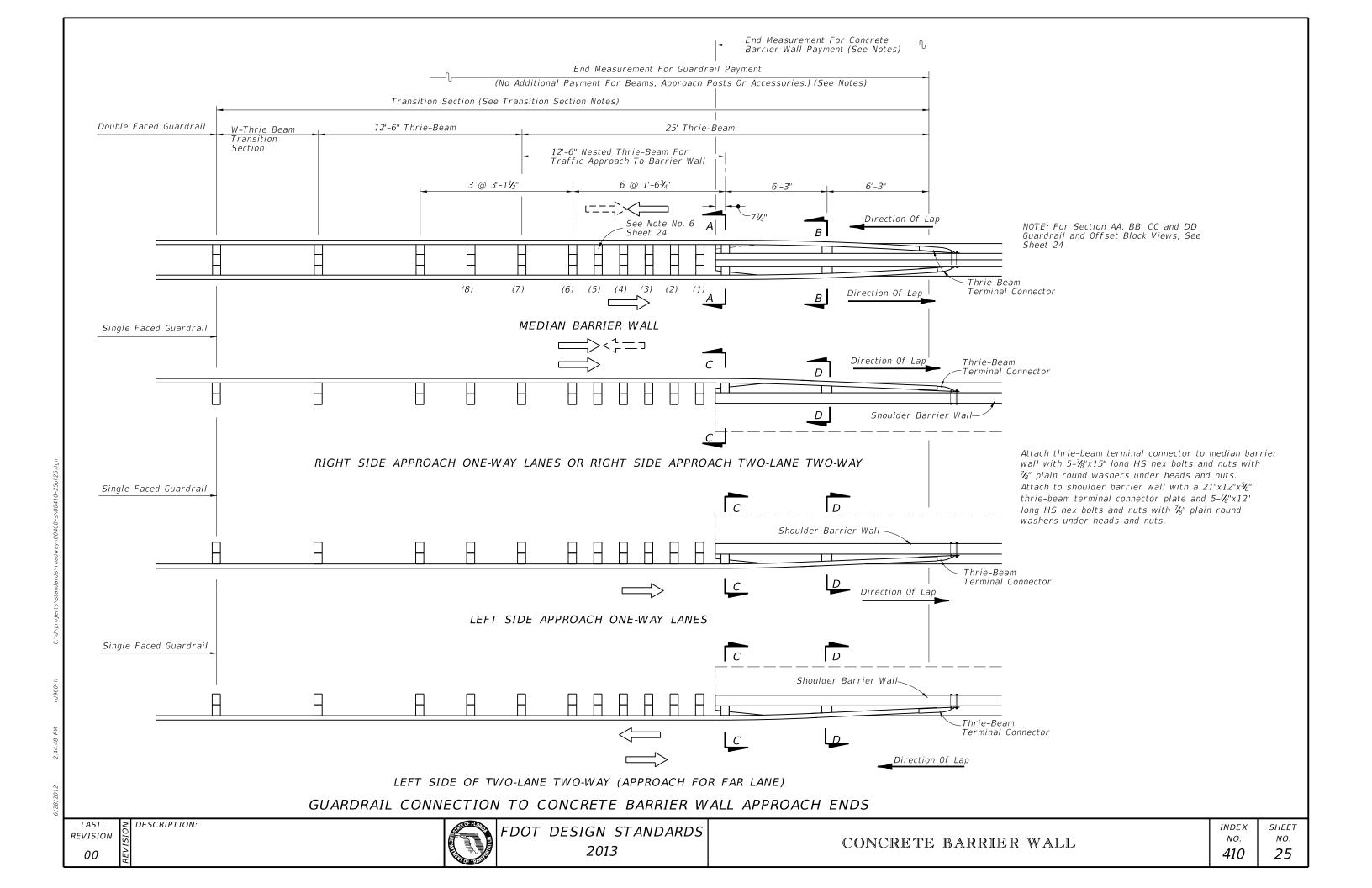
DESCRIPTION:

½" Expansion Material — (Footing Isolation Same)

¾" Chamfer∕

Face Of Wall

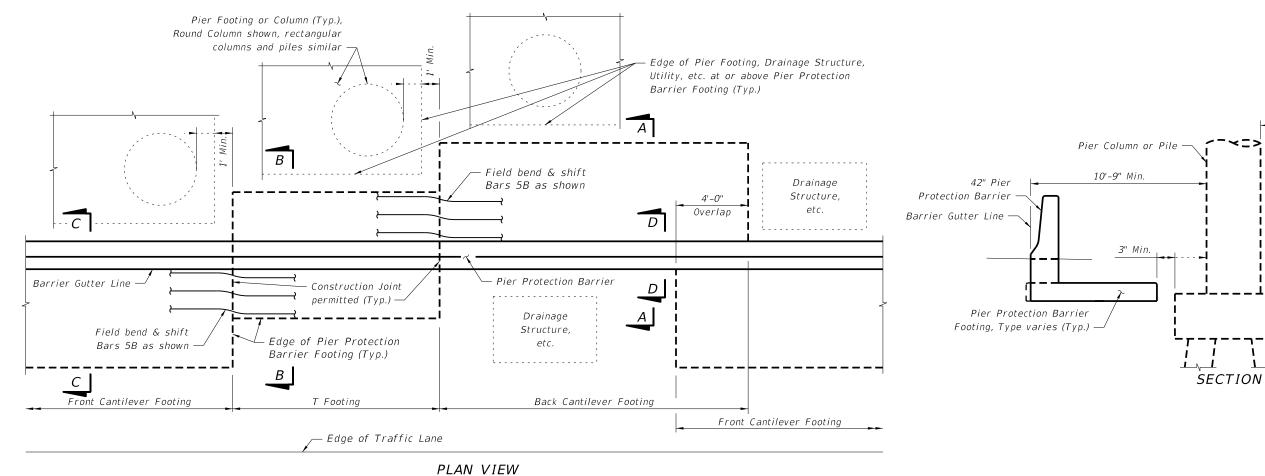




GENERAL NOTES

- 1. Concrete shall be Class III or IV unless otherwise called for in the plans. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Specifications, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finished Coating in accordance with Section 400 only when called for in the plans.
- 2. Construct Pier Protection Barrier continuous without transverse contraction or expansion joints. Transverse construction joints may be used at a spacing greater than or equal to 40°. Provide longitudinal reinforcing steel continuous across construction joints.
- 3. When the Pier Protection Barrier is installed adjacent to Roadway or Shoulder pavement, compact the top 12" of the subgrade to at least 98% of the maximum density determined by FM 1-T 180, Method D.
- 4. Isolate Barrier Wall Inlets, Index 218, from Pier Protection Barriers and Footings with 1" expansion material.

- 5. On roadways designated for reverse laning, mark all downstream barrier ends that are not shielded or outside the clear zone with Type 3 Object Markers. Include the cost of the Object Marker in the cost of the Pier Protection
- 6. Payment: Pier Protection Barrier and Crash Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder 42"), LF, or Shoulder Concrete Barrier Wall (Rigid-Shoulder 54"), LF.
- 7. Provide 3/8" deep crack control V-grooves at 15 to 30' spacing. Locate V-grooves above any joint or discontinuity in the barrier footing. Align V-Grooves perpendicular to the longitudinal axis of the Pier Protection Barrier and make continuous across the top surface and both side faces. For slip formed barriers, score 3/8" V-Grooves while the concrete is still plastic, otherwise pre-form the joints when stationary forms are utilized.



PIER PROTECTION BARRIER FOOTING LAYOUT SCHEMATICS

REVISION 01/01/12

DESCRIPTION:



FDOT DESIGN STANDARDS 2013

PIER PROTECTION BARRIER

INDEXSHEET NO. NO.

411

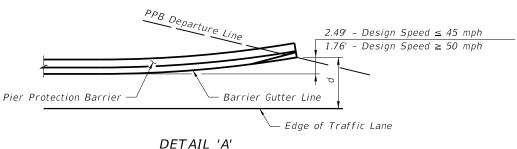
1'-9" Min > 2'-0" Preferred

Pier Footing

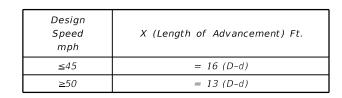
(Varies)

Protection Barrier

Barrier Gutter Line



(Guardrail not shown for clarity)



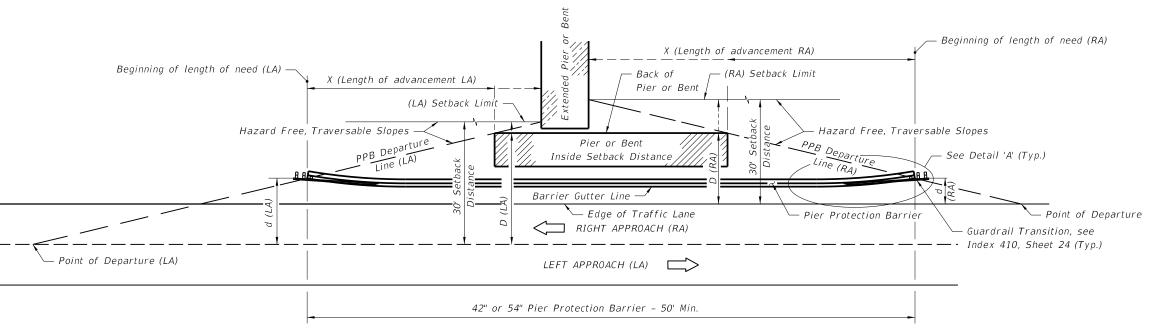
NOTE:

Length of Advancement determined from the diagrams and equations shown establishes the location of the upstream beginning length of need for a Pier Protection Barrier, however, the Length of Advancement for the combination of Pier Protection Barrier and required guardrail can be no less than that required by other details of Index 400.

Equation Variables:

D= Distance in feet from the near edge of the near approach traffic lane to either (a) the back of pier, when the pier is located inside the Setback Distance or (b) the Setback Distance, when the pier extends to or goes beyond the Setback Distance. For left side piers on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane.

d=Distance in feet from the near edge of the near approach traffic lane to the Pier Protection Barrier gutter line at its intersection with the departure line or the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane.



TWO-LANE TWO-WAY TRAFFIC

NOTE:
See Index 400 for Clear
Zone and Horizontal Clearance
Length of Advancement Diagrams.

PPB = Pier Protection Barrier

LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH GUARDRAIL CONTINUATION

LAST REVISION V

DESCRIPTION:



FDOT DESIGN STANDARDS
2013

PIER PROTECTION BARRIER

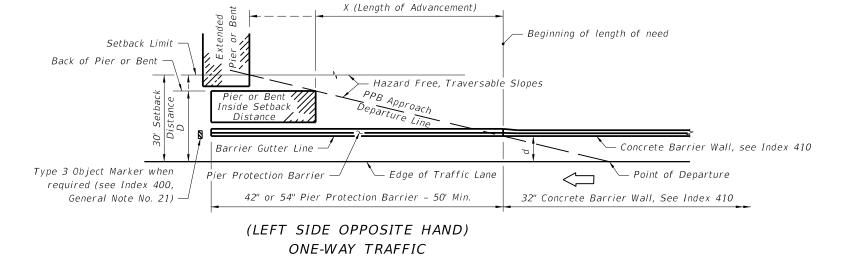
INDEX SHEET NO. NO.

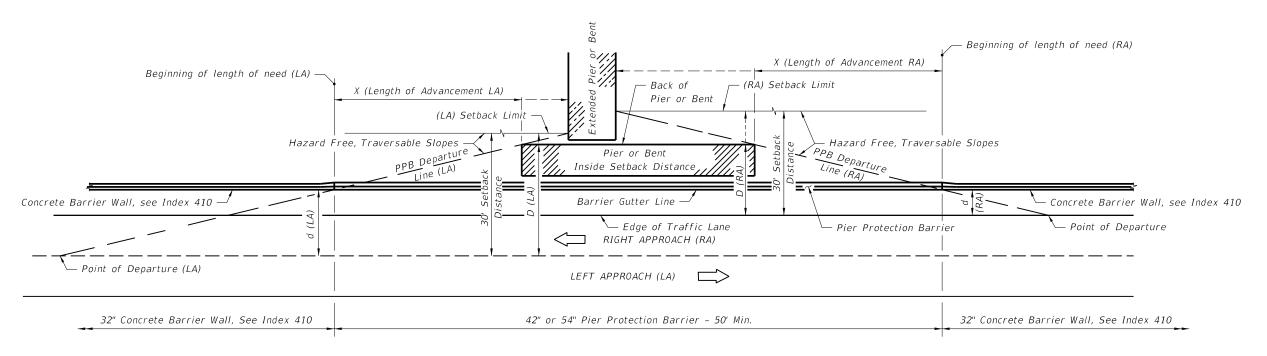
REVISION

07/01/06

DESCRIPTION:







TWO-LANE TWO-WAY TRAFFIC

NOTES: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams. PPB = Pier Protection Barrier

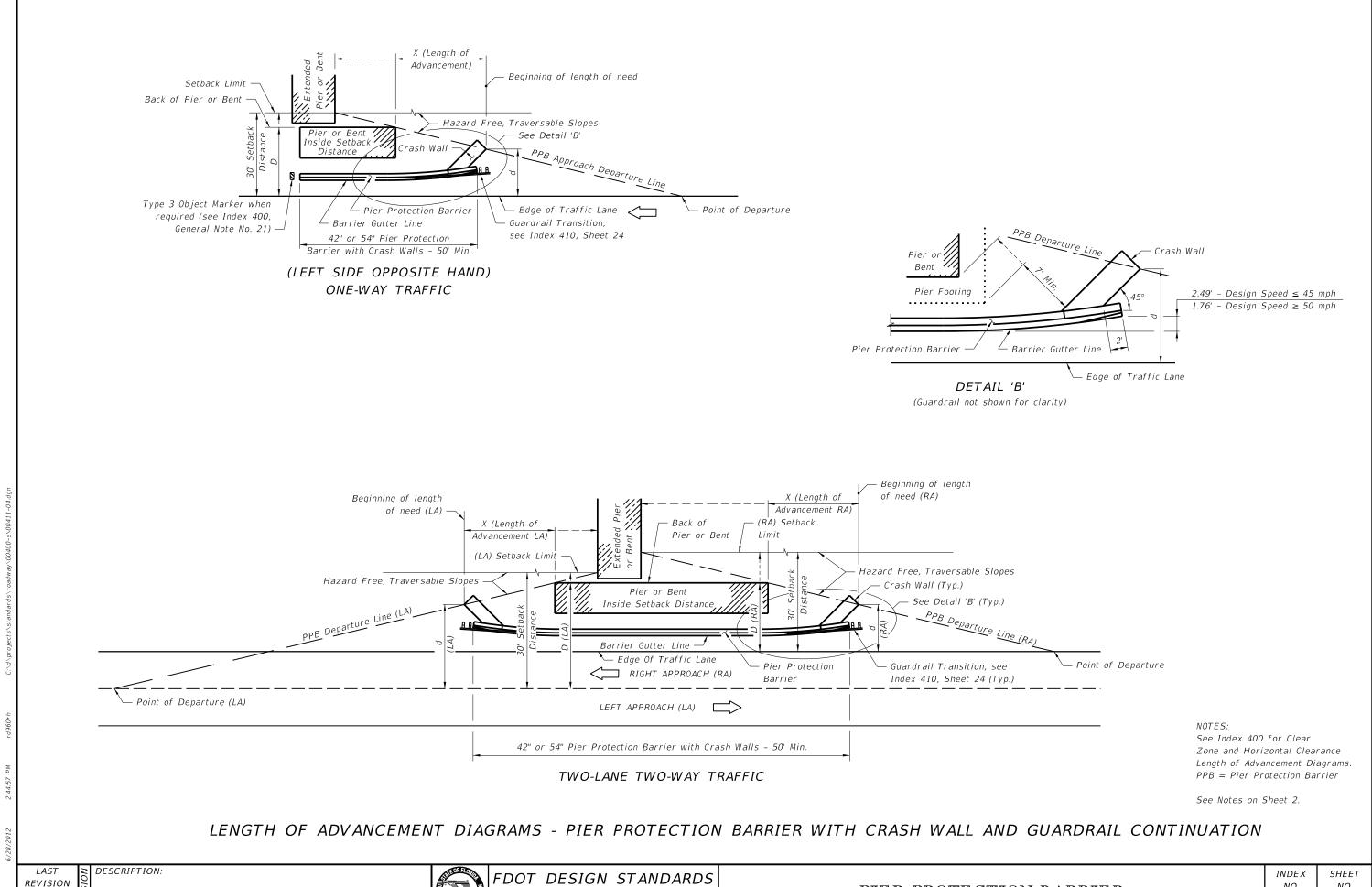
See Notes on Sheet 2.

LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH CONCRETE BARRIER WALL CONTINUATION

SHEET

NO.

3



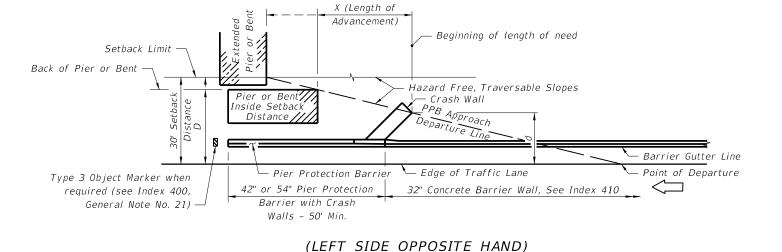
LAST

REVISION

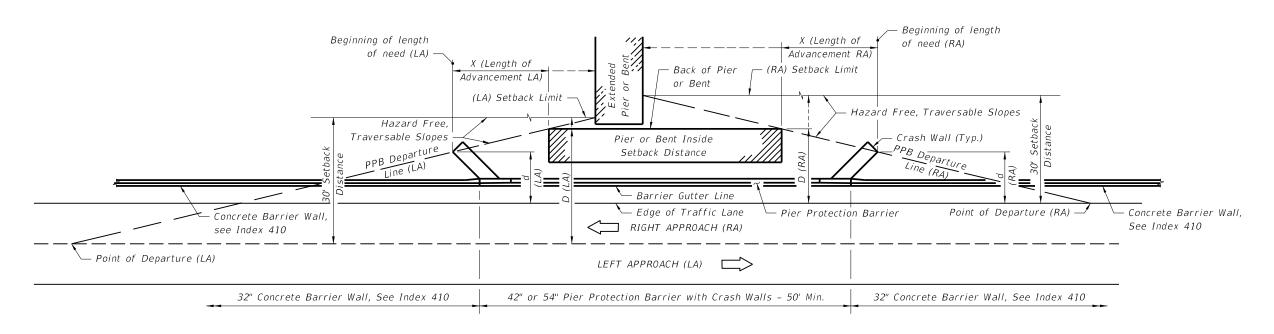
07/01/06

DESCRIPTION:





ONE-WAY TRAFFIC

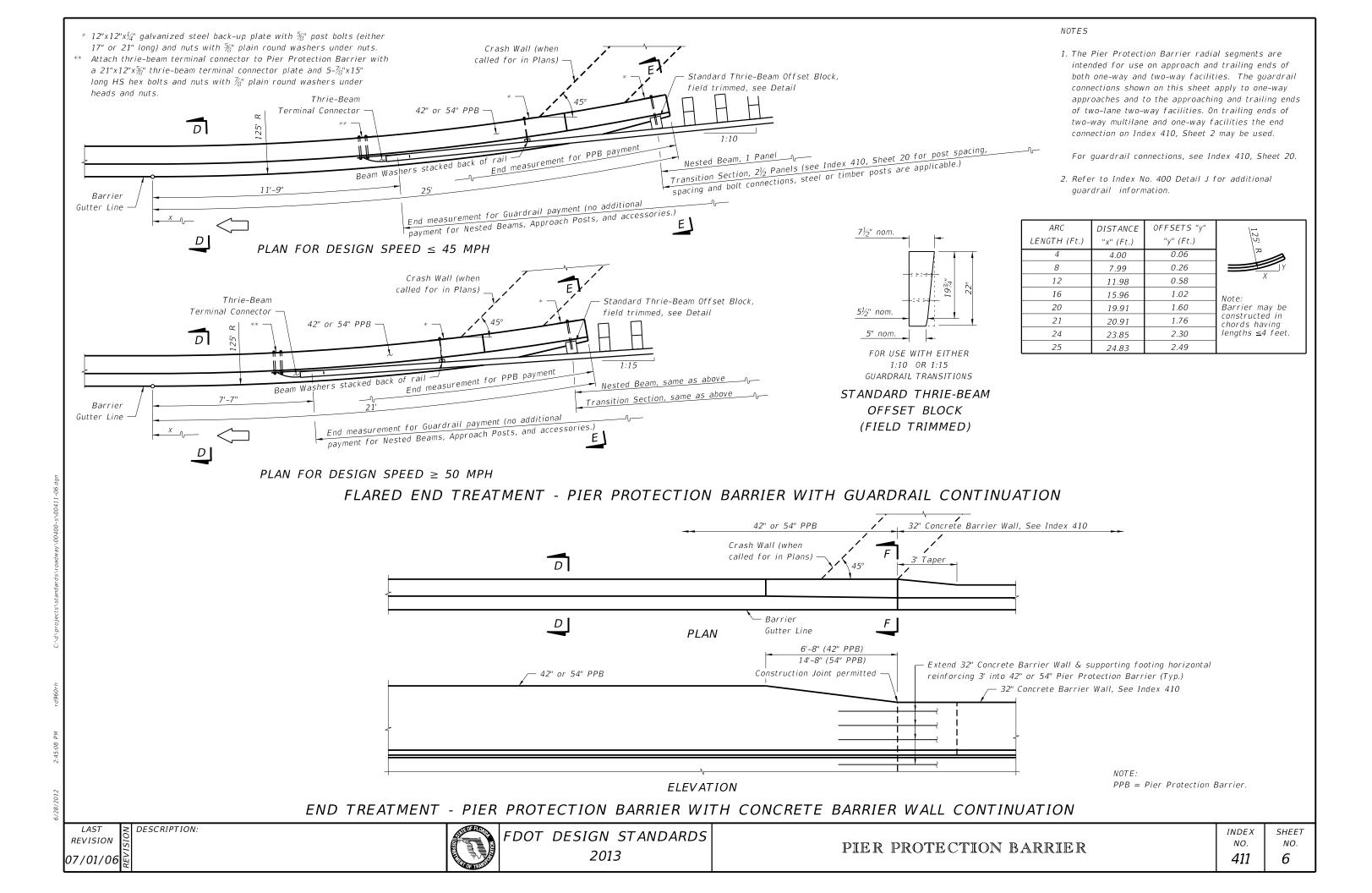


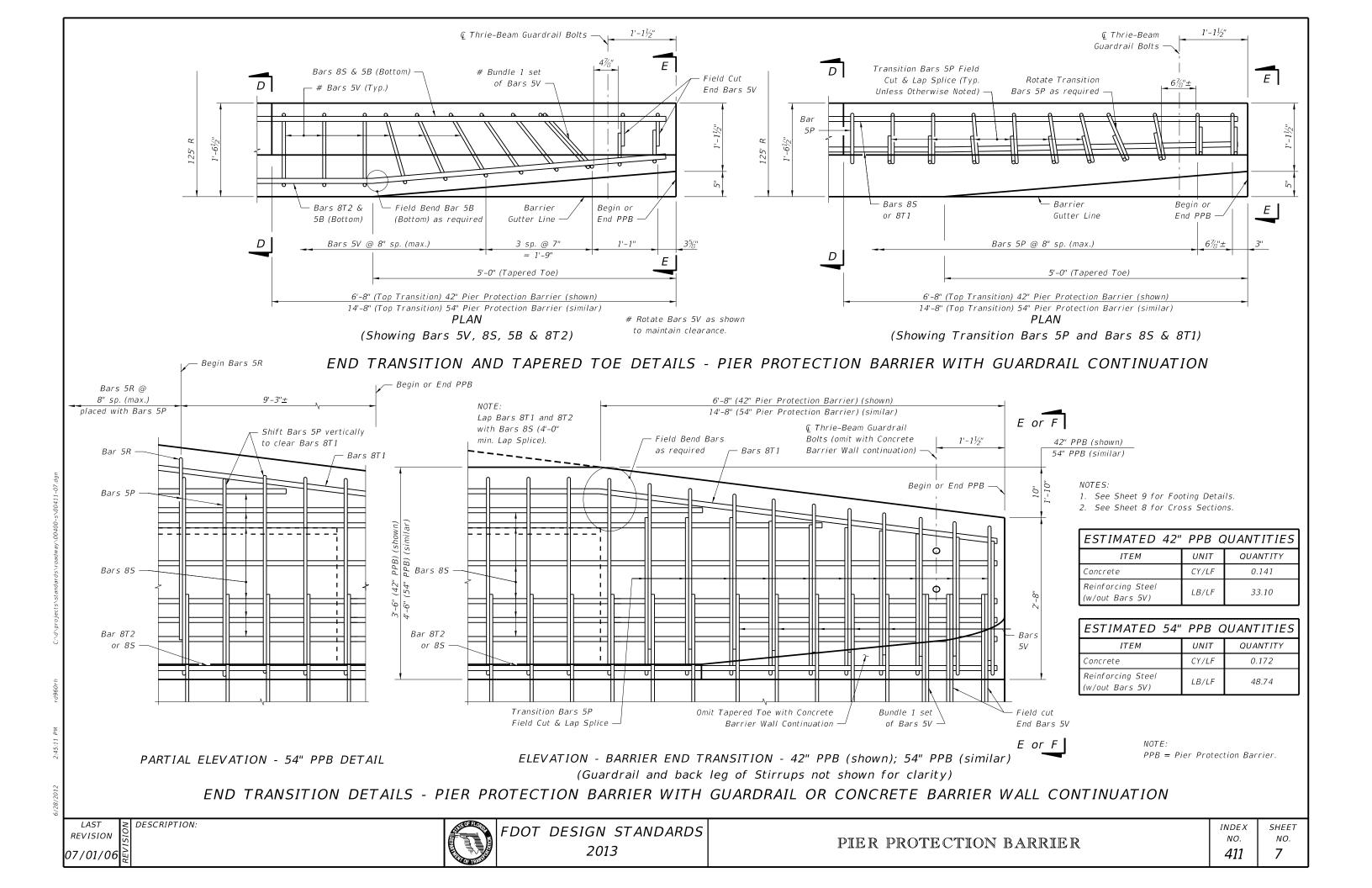
TWO-LANE TWO-WAY TRAFFIC

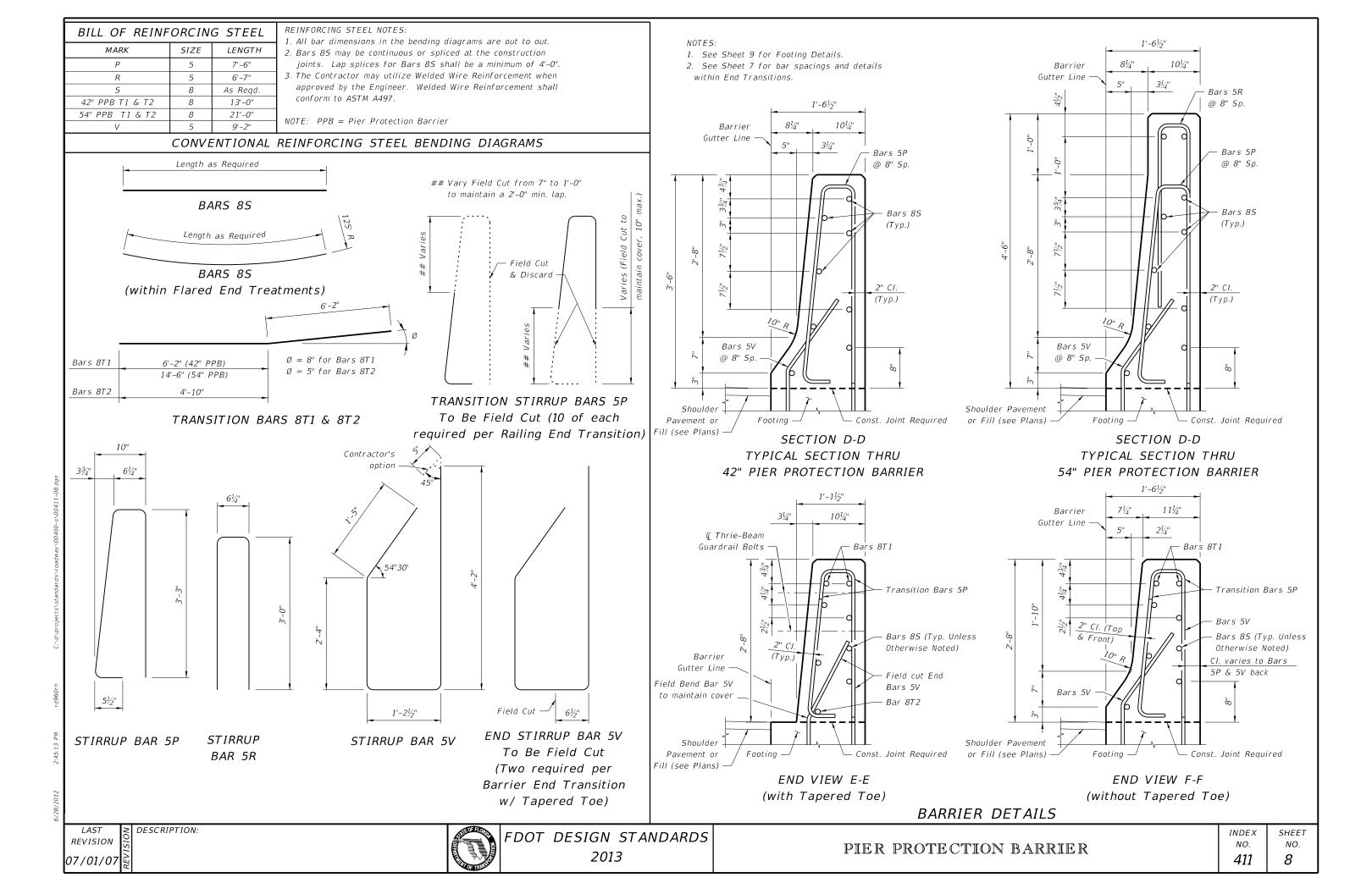
NOTES: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams. PPB = Pier Protection Barrier

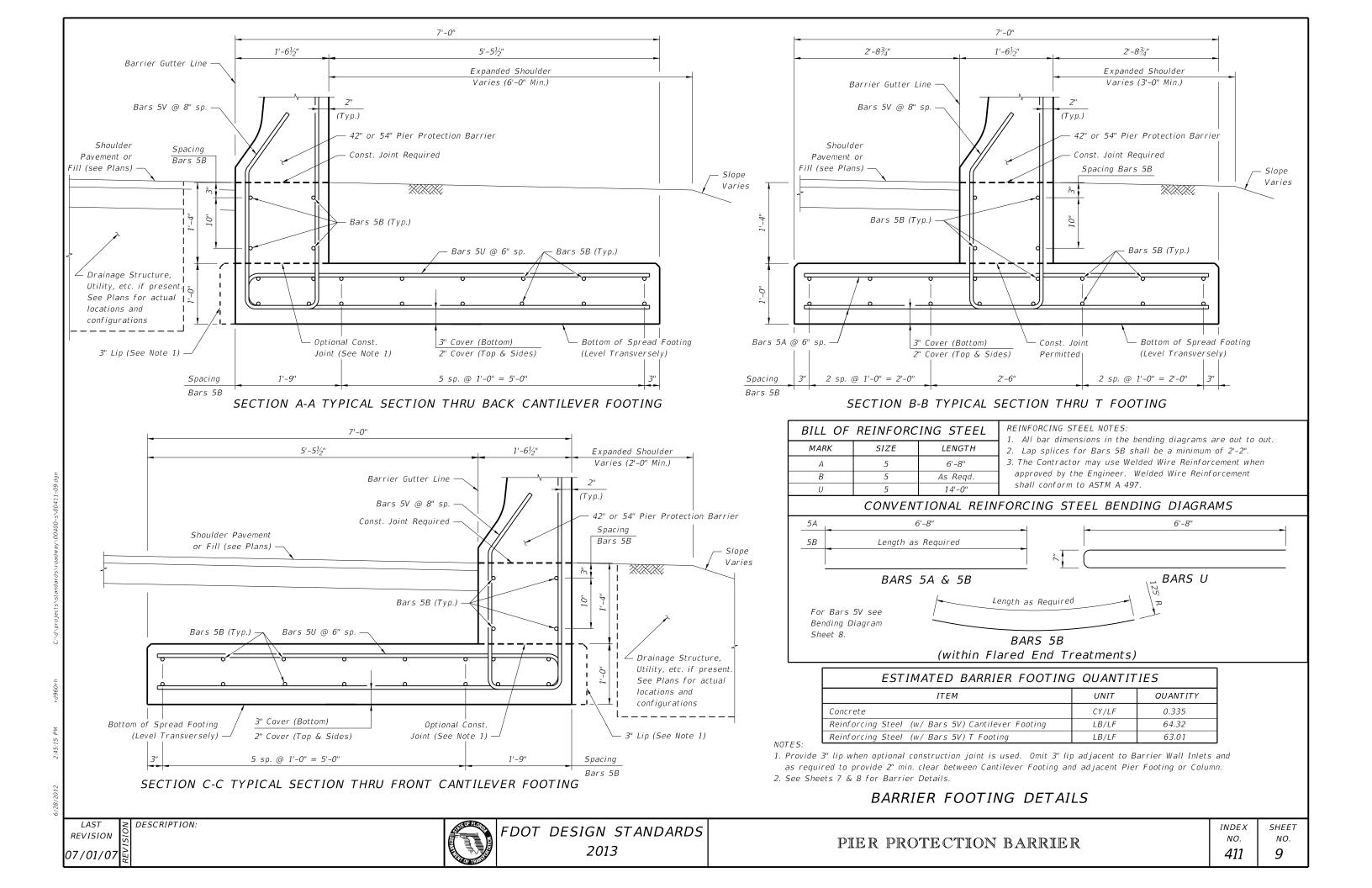
See Notes on Sheet 2.

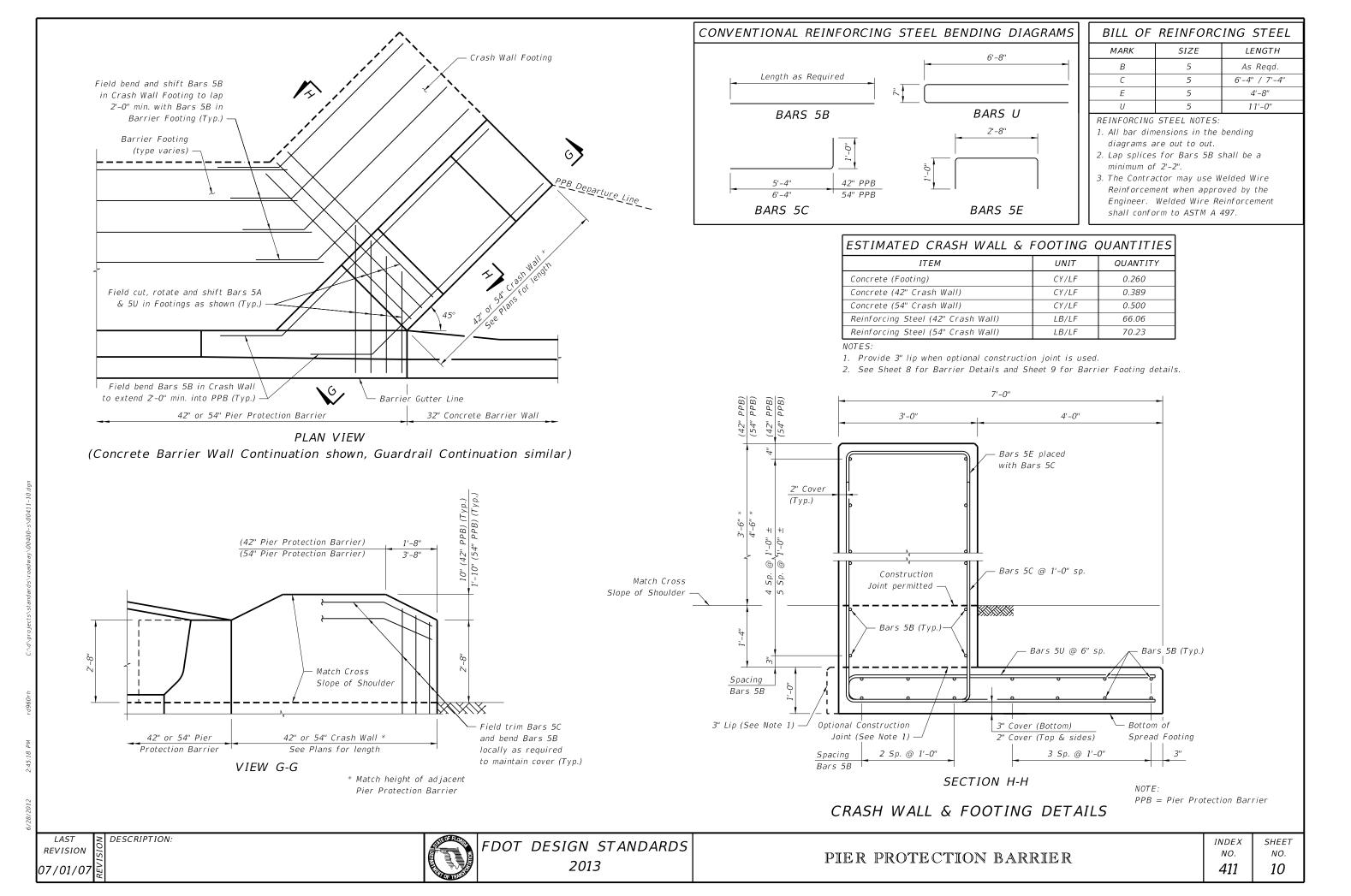
LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH CRASH WALL AND CONCRETE BARRIER WALL CONTINUATION

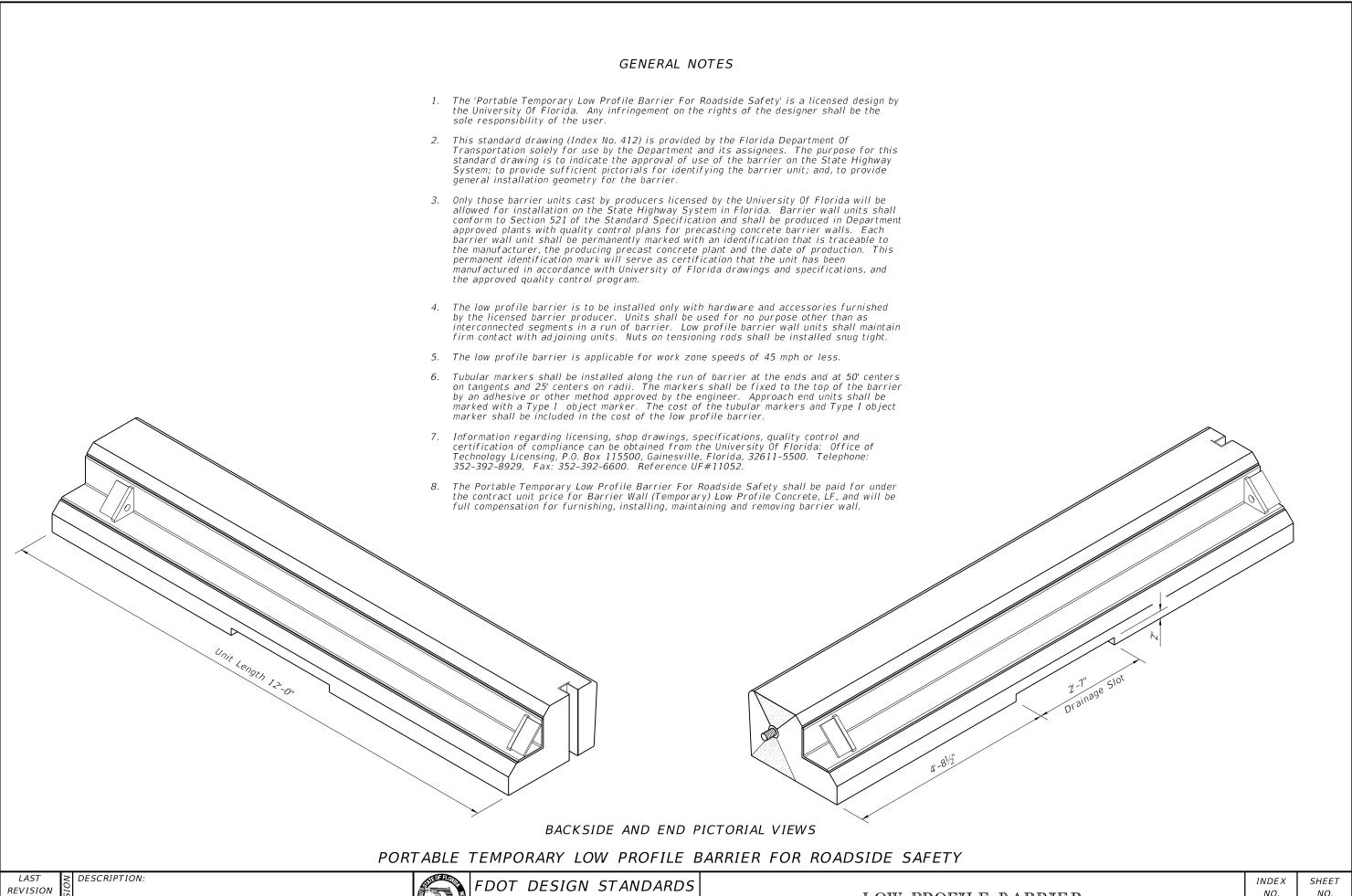




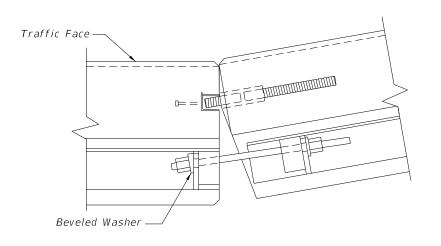




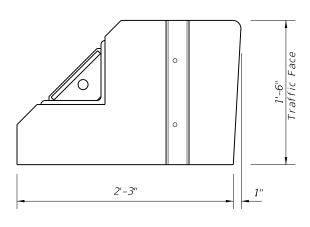




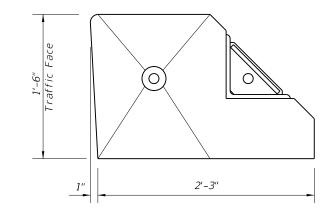
01/01/12



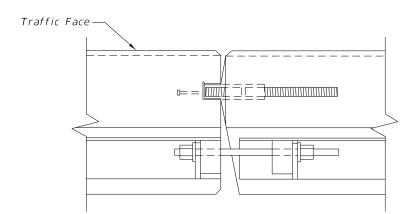
CONCAVE CONNECTION



FLAT FACE FEMALE END

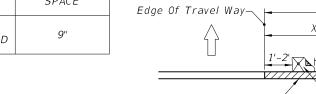


BEVELED FACE MALE END



PARALLEL CONNECTION

WORK POSTED SPEED OFFSET TO TRAVELWAY DEFLECTION SPACE 45 MPH OR LESS 1' MIN, 2' PREFERRED 9"



Flexible or Rigid Pavement or Asphalt Pad

* Minimum 9" on 1:10 or flatter slopes for 'Portable Temporary Low Profile Barrier For Roadside Safety.' For values A, B, D and X see Index No. 600.

END VIEWS

Clear Zone (CZ)

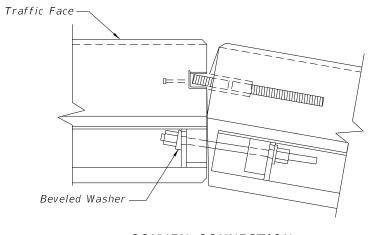
Portable Temporary Low Profile Barrier For Roadside Safety

Notes:

LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement or on an asphalt pad as shown.

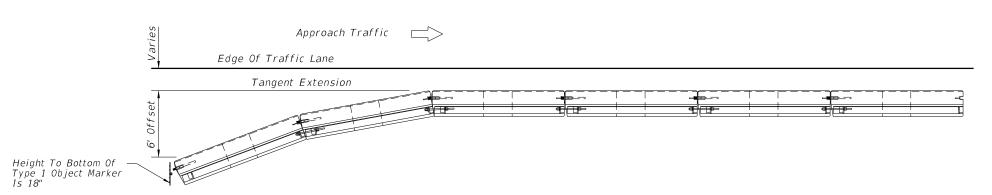
ASPHALT PAD: Where exisiting pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

DEFLECTION SPACE AT DROPOFFS



CONVEX CONNECTION

PLAN VIEWS OF CONNECTIONS



PLAN VIEW OF APPROACH END OFFSET

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

REVISION 01/01/12

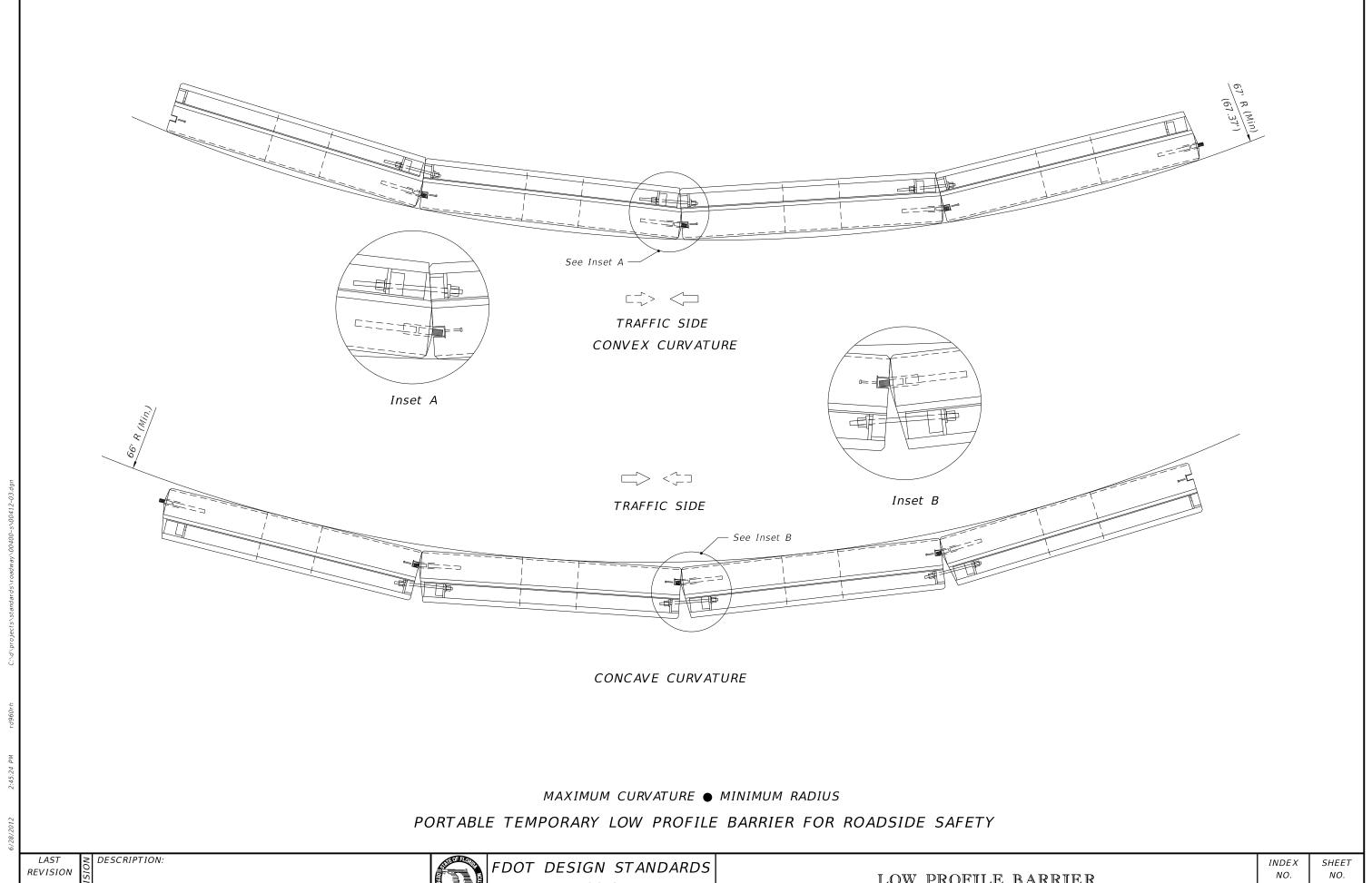
DESCRIPTION:

FDOT DESIGN STANDARDS 2013

LOW PROFILE BARRIER

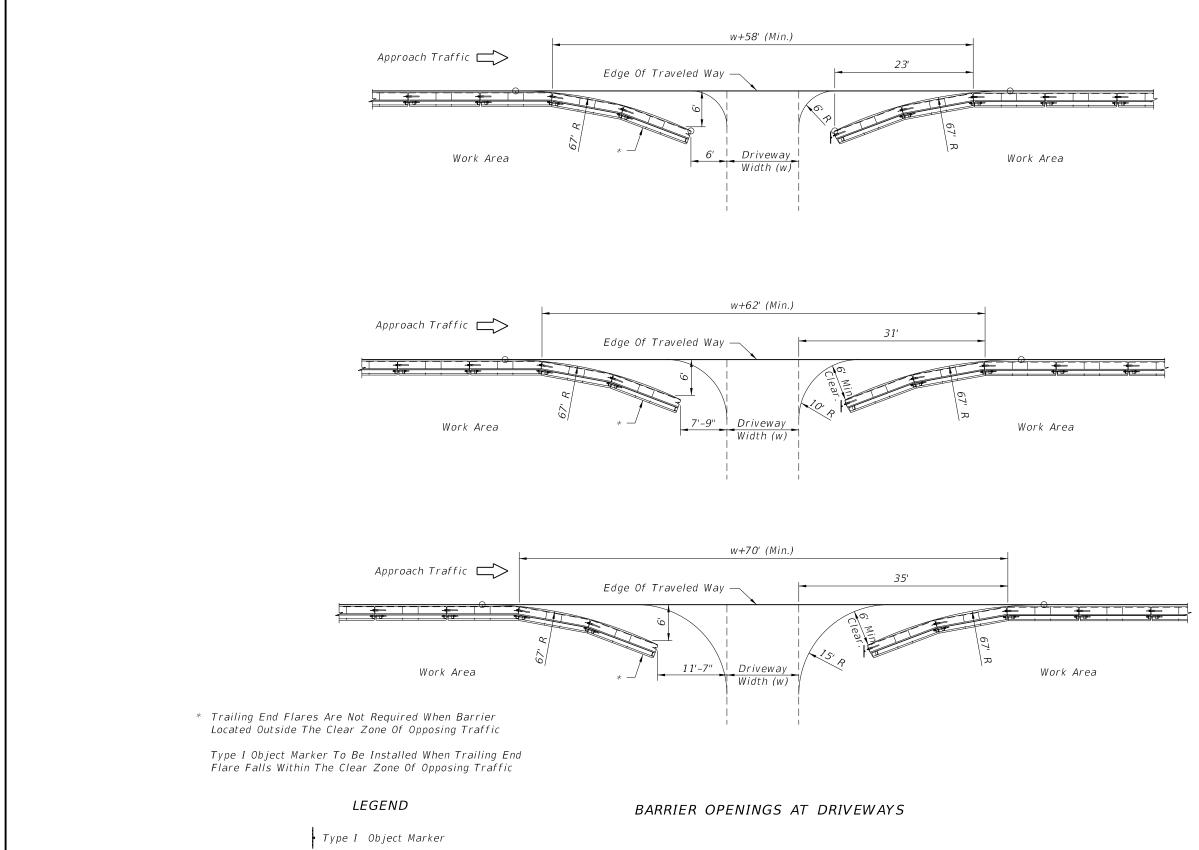
INDEXNO. 412

SHEET NO.



07/01/05





01/01/12

LAST DESCRIPTION: REVISION



FDOT DESIGN STANDARDS 2013

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

INDEXNO. 412

SHEET NO. 4

* Trailing End Flares Are Not Required When Barrier Located Outside The Clear Zone Of Opposing Traffic

Type I Object Marker To Be Installed When Trailing End Flare Falls Within The Clear Zone Of Opposing Traffic

LEGEND

Type I Object Marker

BARRIER OPENINGS AT DRIVEWAYS

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

LAST REVISION 01/01/12

DESCRIPTION:

FDOT DESIGN STANDARDS 2013

LOW PROFILE BARRIER

INDEXNO. 412

SHEET NO. 5

07/01/12

DESCRIPTION:

TO PLOT

The Type K Temporary Concrete Barrier System has been crash tested to NCHRP Report 350 TL-3 criteria or structurally evaluated to meet the requirements of NCHRP Report 350 TL-3 criteria for the installation configurations as shown utilizing the types, sizes, lengths, shapes, strengths and grades of the fabrication and installation materials as shown.

In order to maintain crashworthiness of the system, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

FABRICATION NOTES:

- FABRICATOR PREQUALIFICATION: The Barrier Units shall be made in a prestressed concrete plant that meets the requirements of Specification Section 450 or in a precast plant meeting the requirements of Specification Section 105.
- CONCRETE: Concrete shall be Class IV in accordance with Specification Section 346. Specification Sections 346–10.2 through 346–10.4 are not applicable. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.
- REINFORCING STEEL: All reinforcing steel shall be ASTM A 615, Grade 60 except for Bars 6D1, 6D2 and 6D3. Bars 6D1, 6D2 and 6D3 shall be ASTM A 706 except that a 2¾" diameter pin must be used for the 180 degree bend test. After fabrication, all or part of Bars 6D shall be hot dip galvanized in accordance with Specification Section 962 or coated with a cold galvanizing compound in accordance with Specification Section 975. The minimum limit of galvanizing or coating is shown in the Bending Diagrams. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated. Install Bars 6D within ½" of the plan dimensions. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.

At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with ASTM A 497 and the details shown on Sheet 2 may be utilized in lieu of Bars 4A and 5B.

All dimensions in the Bending Diagrams are out to out. All reinforcing steel shall have a 2" minimum cover except as noted.

- LIFTING SLEEVE ASSEMBLY: Inclusion of the Lifting Sleeve Assemblies is optional. Steel for Pipe Sleeve shall be in accordance with ASTM A 53. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.
- SURFACE FINISH: Construct Barrier Units in accordance with Specification Sections 400 and 521. Finish the top and sides of the Barrier Units with a General Surface Finish. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish. Use stationary metal forms or stationary timber forms with a form liner.

MARKING: Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall. Ink stamps are not allowed. Permanently mark with the following information:

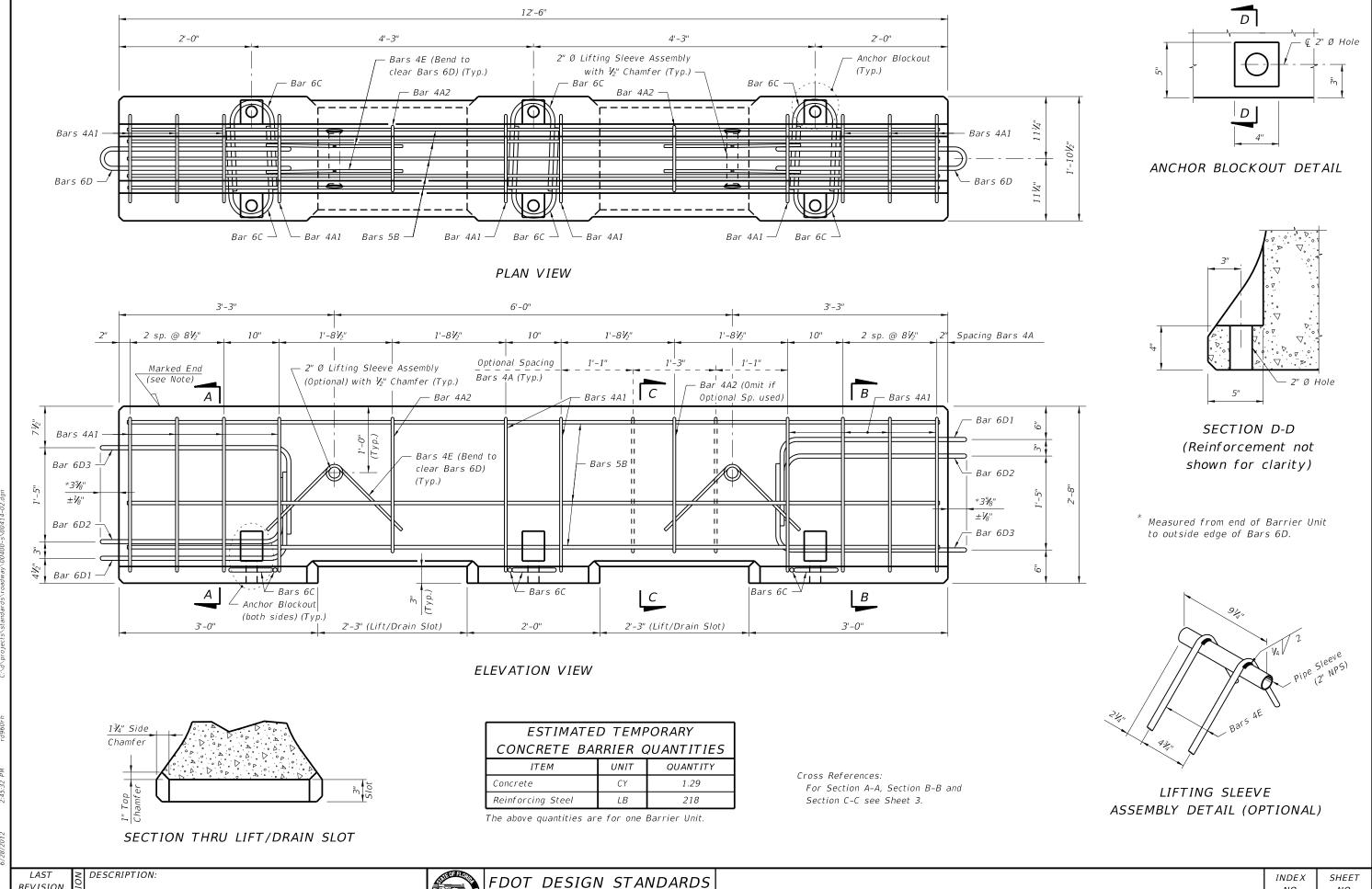
- Type K1
- Fabricator's name or symbol
- Date of manufacture (day, month and year)

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

Alternate Designs: Manufacturers/vendors seeking approval of proprietary Temporary Barrier Systems for inclusion on the Qualified Products List (QPL) as alternative designs shall submit a Product Application package. The application package shall include manufacturer's product drawings, specifications, installation manual, National Cooperative Highway Research Program (NCHRP) Report 350 or Manual for Assessing Safety Hardware (MASH) Test Level 3 (TL-3) crash test documentation and the FHWA "Letter of Acceptance." The posted QPL drawings will need to include the following:

- 1. Anchorage, bolting, and staking details for connections to asphalt and concrete pavement.
- 2. Sections and tables showing required deflection space and minimum offsets to above ground hazards or drop-offs.
- 3. Alignment and length of need details.
- 4. Transition and overlap details.
- 5. End treatment details.

FDOT DESIGN STANDARDS
2013

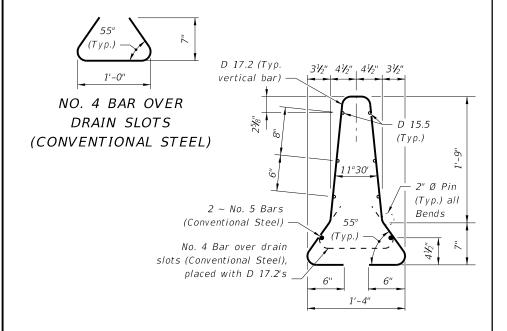


2013

REVISION

07/01/07

ALTERNATE REINFORCING STEEL DETAIL WELDED WIRE REINFORCEMENT

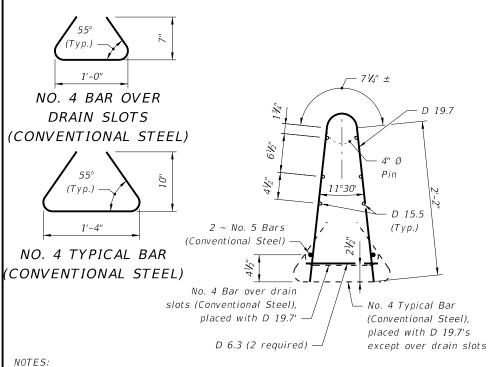


NOTES:

Place 2 ~ No. 5 Bars (12'-3" long) in bottom of Welded Wire Reinforcement cage as shown.

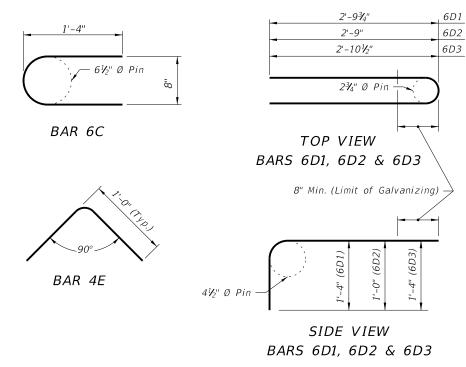
D 17.2 spacing shall match spacings for Bars 4A shown in Elevation View, Sheet 2. Field trim D 17.2's to clear drain slots by 2".

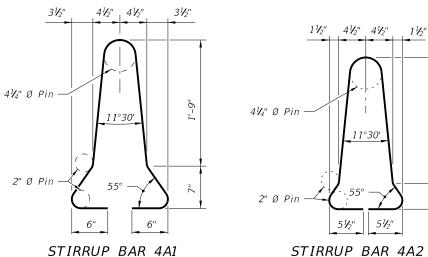
CONFIGURATION ONE

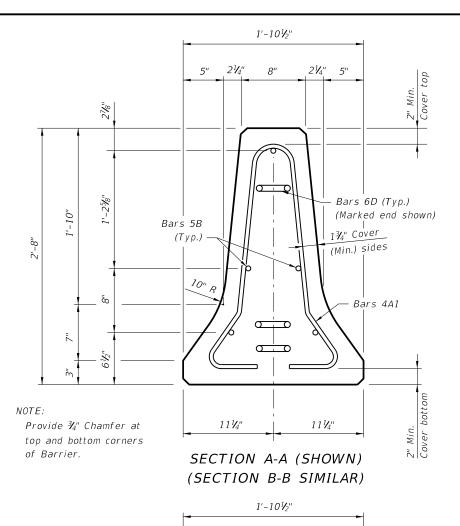


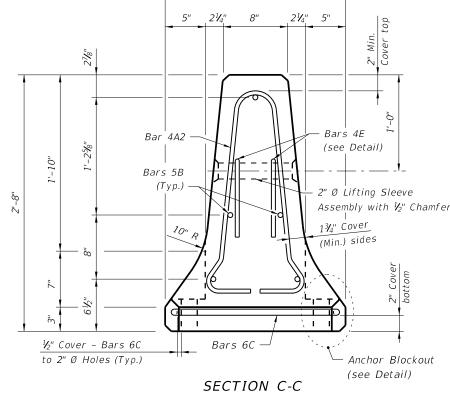
CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL				
MARK	SIZE	NUMBER	LENGTH	
A1	4	10	6'-1"	
A2	4	2	5'-5"	
В	5	5	12'-3" (Straight)	
С	6	6	3'-1"	
D1	6	2	8'-4"	
D2	6	2	7'-6"	
D3	6	2	8'-6"	
E	4	4	2'-0"	









(Bars 6D not shown for clarity)

CONFIGURATION TWO

D 19.7 spacing shall match spacings for Bars 4A shown in Elevation View,

Place 2 ~ No. 5 Bars (12'-3" long) in bottom of Welded Wire

Sheet 2. Field trim D 19.7's to clear drain slots by 2".

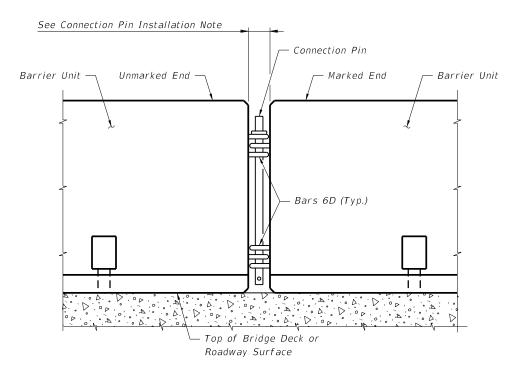
LAST REVISION 07/01/07

Reinforcement cage as shown.

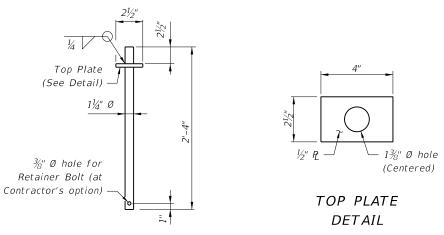
DESCRIPTION:

FDOT DESIGN STANDARDS 2013





DETAIL OF CONNECTION BETWEEN BARRIER UNITS



CONNECTION PIN DETAIL

NOTES FOR ALL INSTALLATIONS:

LIMITATION OF USE: This Temporary Concrete Barrier System is intended for work zone traffic control and other temporary applications. It shall not be used for permanent traffic railing construction unless specifically permitted by the Plans. Except as shown for the Back Filled Roadway Installations, the Barrier Units must be installed on a flexible pavement (asphalt) or rigid pavement (concrete) surface as shown with a cross slope of 1:10 or flatter. Except as shown for transition installations, Type K Barrier Units are not intended to be bolted down or staked down in locations where they can be impacted from the back side.

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

SURFACE PREPARATION: Except as shown for the Back Filled Roadway Installations, remove all debris, loose dirt and sand from the pavement, bridge deck or Asphalt Pad surface within the barrier footprint just prior to placement of the Barrier Units.

CONNECTION PIN ASSEMBLY: Steel for Connection Pin and Top Plate assemblies shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Nondestructive testing of welds shall not be required. At the Contractor's option, a $\frac{2}{3}$ " diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.

CONNECTION PIN INSTALLATION: Initially set Barrier Units by using a 35% wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 5). Barrier Units shall not be used unconnected.

DELINEATION: Mount Type C Steady-Burn Lights on top of Barrier Units that are used as traffic barriers along travel ways in work zones. Space the lights at 50' centers in transitions, 100' centers on curves and 200' centers on tangent alignments. Refer to "Warning Lights" on Index No. 600 for additional information.

REUSE OF UNITS: Barrier Units may be reused provided they have the structural integrity and surface qualities of new units. Do not use Barrier Units without Marking Plates.

REUSE OF CONNECTION PINS: Connection pins may be reused if they have the structural integrity of new pins.

INSTALLATIONS ON CURVED ALIGNMENTS: The details presented in these Standards are shown for installations on tangent alignments. Details for horizontally curved alignments are similar.

TRANSITIONS: Transitions are required between freestanding, bolted down, staked down and back filled Type K Barrier installations, see Sheet 8 for transition requirements and details. Transitions are also required between installations of Type K Barrier and other types of temporary barrier, see Index No. 415 for transition requirements and details. Splices and transitions are required between installations of Type K Barrier and permanent Bridge or Roadway Traffic Railings, see Sheets 9 through 13 for transition requirements and details. Transitions are required between installations of Type K Barrier and Proprietary (QPL) Barrier Systems, See Sheets 14 and 15 for transition requirements and details.

PAYMENT: Barrier Units for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary) (F&I) (Type K), LF. Any relocation of the Barrier Units required for the project shall be paid for under the contract unit price for Barrier Wall (Temporary) (Relocate) (Type K), LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights (Temp. Barrier Wall Mount) (Type C, Steady Burn), ED. The Contractor shall furnish Barrier Units except when the Plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return. Unless otherwise noted on the Plans, the BarrierUnits shall become the property of the Contractor and shall be removed from the site prior to acceptance of the completed project.

NOTES FOR THRIE BEAM GUARDRAIL SPLICE INSTALLATIONS:

THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices in accordance with AASHTO M 180, Type II (Zinc coated) and as follows:

Two panels per splice (One panel per side) of Class B (10 Gauge), or

Four panels per splice (Two nested panels per side) of Class A (12 Gauge).

Guardrail panel length shall be 12'-6". Provide and install all other associated metallic guardrail components (Terminal Connectors, Shoulder Bolts, Hex Bolts and Nuts, Filler Plates, etc.) in accordance with Index No. 400.

Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted. Do not drill or cut through utilities or conduits within permanent concrete traffic railings.

GUARDRAIL OFFSET BLOCKS: Provide and install timber Offset Blocks meeting the material requirements of Index No. 400. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.

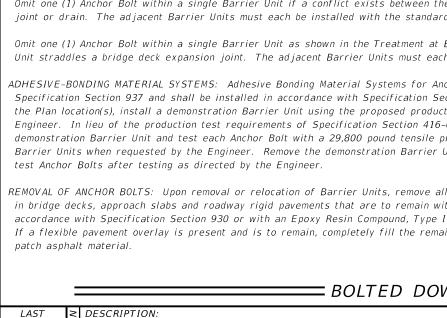
CONCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification Section 346, any Class, or a commercially available prebagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification Section 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.

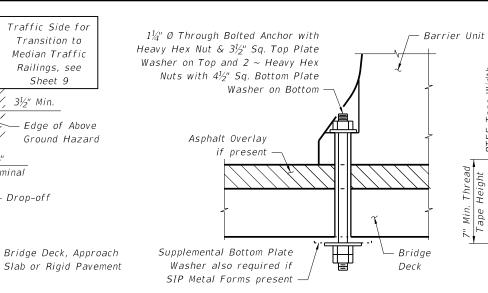
DESCRIPTION: LAST REVISION 01/01/11



REVISION

01/01/08





THROUGH BOLTED ANCHOR

INSTALLATION ON BRIDGE DECK

OR RIGID PAVEMENT SIMILAR: INSTALLATION ADJACENT TO DROP-OFF SHOWN, MEDIAN TRANSITION INSTALLATION SIMILAR)

Traffic Side for

Transition to

Median Traffic

Railings, see

- Edge of Above Ground Hazard

31/3" Min.

- Drop-off

NOTES FOR BOLTED DOWN BRIDGE, APPROACH SLAB, ROADWAY AND TRANSITION INSTALLATIONS:

TYPICAL SECTION (BRIDGE DECK SHOWN, APPROACH SLAB

Barrier Unit

TRAFFIC

SIDE

See Anchor Bolt Detail

Asphalt Overlay

if present

LIMITATION OF USE: This installation technique can only be used on rigid pavement and concrete bridge decks as shown. Barrier Units shall not be bolted down on bridge superstructures that contain post-tensioned tendons within the concrete deck (top flange of concrete box girders) or on bridge superstructures consisting of longitudinally prestressed, transversely post-tensioned, solid or voided concrete slab units. Anchor Bolts must not be installed on both sides of the Barrier Units. Do not bolt down Barrier Units across bridge finger or modular expansion joints.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchor Bolts shall be fully threaded rods in accordance with ASTM F 1554 Grade 36. Anchor Bolts for through bolting shall be in accordance with ASTM A 307 or ASTM F 1554 Grade 36. Nuts shall be in accordance with ASTM A 563 or ASTM A 194. Flat Washers shall be in accordance with ASTM F 436 and Plate Washers shall be in accordance with ASTM A 36 or ASTM A 709 Grade 36.

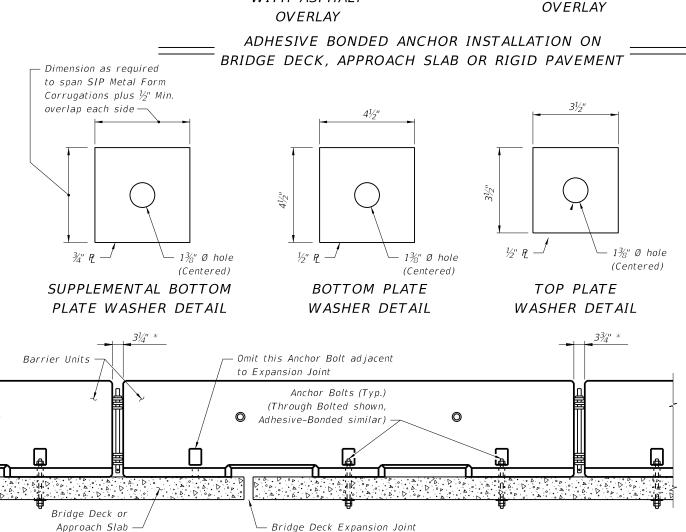
Install three (3) Anchor Bolts per Barrier Unit on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of Anchor Bolts required in Transition Installations see Sheets 8 and 9 and Index No. 415. Drilling through deck reinforcing steel to install Anchor Bolts is permitted. Unless otherwise shown in the Plans, at the Contractor's option Barrier Units may be installed by through bolting (where geometrically possible) or by the use of Adhesive-Bonded Anchor Bolts. Do not drill into or otherwise damage the tops of supporting beams or girders, bridge deck expansion joints or drains. Install Anchor Bolts and Nuts so that the maximum extension beyond the face of the Barrier Units is ½". Snug tighten the Nuts on the Anchor Bolts. For through bolted installations, snug tighten the double Nuts on the underside of the deck against each other to minimize the potential for loosening.

Omit one (1) Anchor Bolt within a single Barrier Unit if a conflict exists between the Anchor Bolt location and a bridge deck expansion joint or drain. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

Omit one (1) Anchor Bolt within a single Barrier Unit as shown in the Treatment at Bridge Deck Expansion Joint Schematic if the Barrier Unit straddles a bridge deck expansion joint. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

ADHESIVE-BONDING MATERIAL SYSTEMS: Adhesive Bonding Material Systems for Anchor Bolts shall be Type HSHV in accordance with Specification Section 937 and shall be installed in accordance with Specification Section 416. Prior to installation of the Barrier Units in the Plan location(s), install a demonstration Barrier Unit using the proposed production installation method, at a location approved by the Engineer. In lieu of the production test requirements of Specification Section 416-6, install six (6) Adhesive-Bonded Anchor Bolts in the demonstration Barrier Unit and test each Anchor Bolt with a 29,800 pound tensile proof load. Install and test additional demonstration Barrier Units when requested by the Engineer. Remove the demonstration Barrier Unit prior to testing the Anchor Bolts. Remove the

REMOVAL OF ANCHOR BOLTS: Upon removal or relocation of Barrier Units, remove all Anchor Bolts and completely fill the remaining holes in bridge decks, approach slabs and roadway rigid pavements that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold



TREATMENT AT BRIDGE DECK EXPANSION JOINT SCHEMATIC

- Barrier Unit

1¼" Ø Adhesive-Bonded

31/3" Sq. Top Plate Washer

Anchor with Heavy Hex Nut &

Embedment

Bridge Deck, Approach Slab or

Roadway Rigid Pavement

* To accommodate movement at Expansion Joint, set

Barrier Units with 33/4" gap at locations shown

See Optional PTFE Taping Detail

WITHOUT ASPHALT

** Threads may be wrapped

removal of anchors.

Asphalt Overlay

OPTIONAL PTFE

TAPING DETAIL

7" Min. **

Embedment

See Optional PTFE

Taping Detail

WITH ASPHALT

with a single overlapping layer

of PTFE tape to facilitate

: $BOLTED\;DOWN\;BRIDGE,\;APPROACH\;SLAB,\;ROADWAY\;AND\;TRANSITION\;INSTALLATIONS <math>=$

SHEET

Barrie

NOTES FOR STAKED DOWN ROADWAY AND TRANSITION INSTALLATIONS:

LIMITATION OF USE: This installation technique can only be used on flexible pavement or an Asphalt Pad as shown. Stakes must not be installed on both sides of the Barrier Units.

ASPHALT PAD: Where existing flexible pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

STAKES: Provide steel for Stake assemblies in accordance with ASTM A 36 or ASTM A 709 Grade 36. All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal shall be E60XX or E70XX. Nondestructive testing of welds is not required.

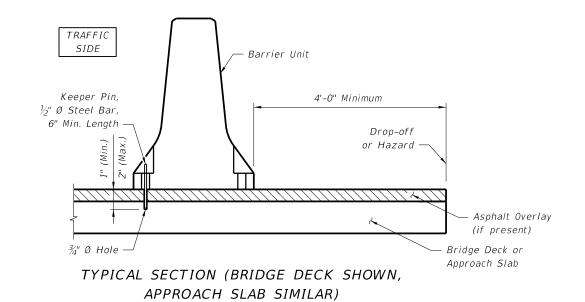
Install three (3) Stakes on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of stakes required in Transition Installations see Sheets 8 and 9 and Index No. 415. Install Stakes so that the Stop Plate is snug against the bottom of the Anchor Blockout.

BURIED UTILITIES: Prior to installation of Stakes verify locations of all adjacent buried utilities, drainage structures, pipes, etc. If conflicts between Stake locations and buried elements exist, a maximum of two (2) Stakes within a single Barrier Unit may be omitted if the adjacent Barrier Units are installed with the standard three (3) Stakes.

REMOVAL OF STAKES: Upon removal or relocation of Barrier Units, completely remove all Stakes and completely fill the remaining holes in flexible pavement that is to remain with hot or cold patch asphalt material.

REUSE OF STAKES: Stakes may be reused if they have the structural integrity of new stakes.

= STAKED DOWN ROADWAY AND TRANSITION INSTALLATIONS =



NOTES FOR FREE STANDING BRIDGE OR APPROACH SLAB INSTALLATIONS:

KEEPER PINS: Keeper Pins shall be ½" diameter, smooth steel bar in accordance with ASTM A 36 or ASTM A 709 Grade 36. As directed by the Engineer in order to limit vibration induced translation of the Barrier Units, install one (1) Keeper Pin per Barrier Unit on the traffic side of the Barrier Units as shown. Do not drill into or otherwise damage bridge deck expansion joints or drains.

REMOVAL OF KEEPER PINS: Upon removal or relocation of Barrier Units, remove all Keeper Pins and completely fill the remaining holes in bridge decks and approach slabs that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.

=FREESTANDING BRIDGE OR APPROACH SLAB INSTALLATIONS=

Edge of Above Ground Hazard or Obstruction Limits of Flexible Pavement or Rigid Pavement or Asphalt Pad TRAFFIC 2'-0" Minimum Design Speed 45 MPH SIDE or Less Barrier Unit 4'-0" Minimum Design Speed 50 MPH and Greater Flexible or Rigid 2'-0" Min. - Design Speed 45 MPH or less Pavement, or 2'-0" Min. - Design Speed 50 MPH Asphalt Pad and greater with drop-off Drop-off → ≤ 4'-0" and no traffic below 4'-0" Min. - All other drop-off conditions with Design Speed 50 MPH and greater

TYPICAL SECTION

NOTES FOR FREE STANDING ROADWAY INSTALLATION:

LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement or on an Asphalt Pad as shown.

ASPHALT PAD: Where existing pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

= FREESTANDING ROADWAY INSTALLATION **=**

LAST REVISION 01/01/11

DESCRIPTION:



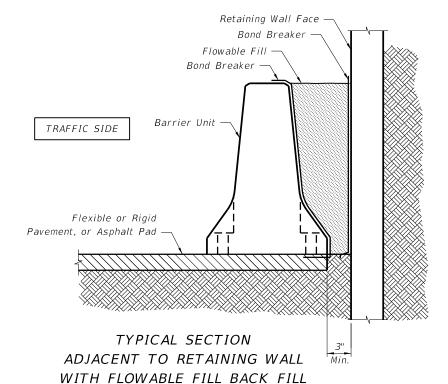
NOTES FOR FREESTANDING MEDIAN INSTALLATION:

KEEPER PINS: Required for Bridge Decks only, Keeper Pins shall be $\frac{1}{2}$ " diameter, smooth steel bar in accordance with ASTM A 36 or ASTM A 709 Grade 36. As directed by the Engineer in order to limit vibration induced translation of the Barrier Units, install one (1) Keeper Pin per Barrier Unit as shown. Alternate Keeper Pin locations from side to side of Barrier Units along the length of the installation. Do not drill into or otherwise damage bridge deck expansion joints or drains. Upon removal or relocation of Barrier Units, remove all Keeper Pins and completely fill the remaining holes in bridge decks and approach slabs that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.

TYPICAL SECTION

ASPHALT PAD: Where existing pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

=FREESTANDING MEDIAN INSTALLATION= (BRIDGE DECK SHOWN, APPROACH SLAB, ASPHALT PAD, FLEXIBLE OR RIGID PAVEMENT SIMILAR)



NOTES FOR FLOWABLE FILL BACK FILLED ROADWAY INSTALLATIONS:

FLOWABLE FILL: Provide Flowable Fill in accordance with Specification Section 121.

=FLOWABLE FILL BACK FILL ROADWAY INSTALLATIONS ===

Geotextile Fabric

Barrier Unit

TRAFFIC SIDE

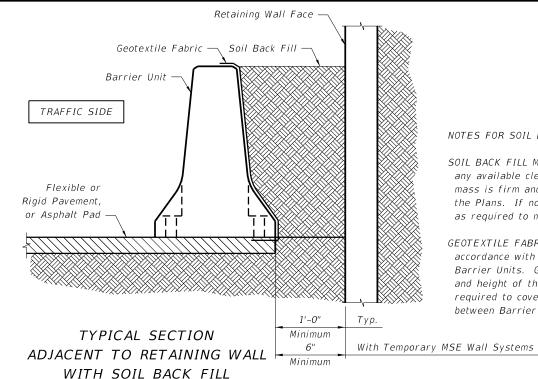
Flexible or

Rigid Pavement,

or Asphalt Pad

5'-0" Min.

Soil Back Fill



NOTES FOR SOIL BACK FILLED ROADWAY INSTALLATIONS:

SOIL BACK FILL MATERIAL: Provide Back Fill Material consisting of any available clean soil. Compact Back Fill Material until the soil mass is firm and unyielding. Provide erosion control as specified in the Plans. If none is specified in the Plans, provide erosion control as required to maintain the integrity of the Back Fill embankment.

GEOTEXTILE FABRIC: Provide Type D-5 Geotextile Fabric in accordance with Index No. 199 to contain Back Fill Material behind Barrier Units. Geotextile Fabric may be continuous over the length and height of the installation or may be individual pieces as required to cover the Lift / Drain Slots and open vertical joints between Barrier Units.

SOIL BACK FILLED ROADWAY INSTALLATIONS =

TYPICAL SECTION WITH SOIL BACK FILL

DESCRIPTION: LAST REVISION 01/01/11



FDOT DESIGN STANDARDS 2013

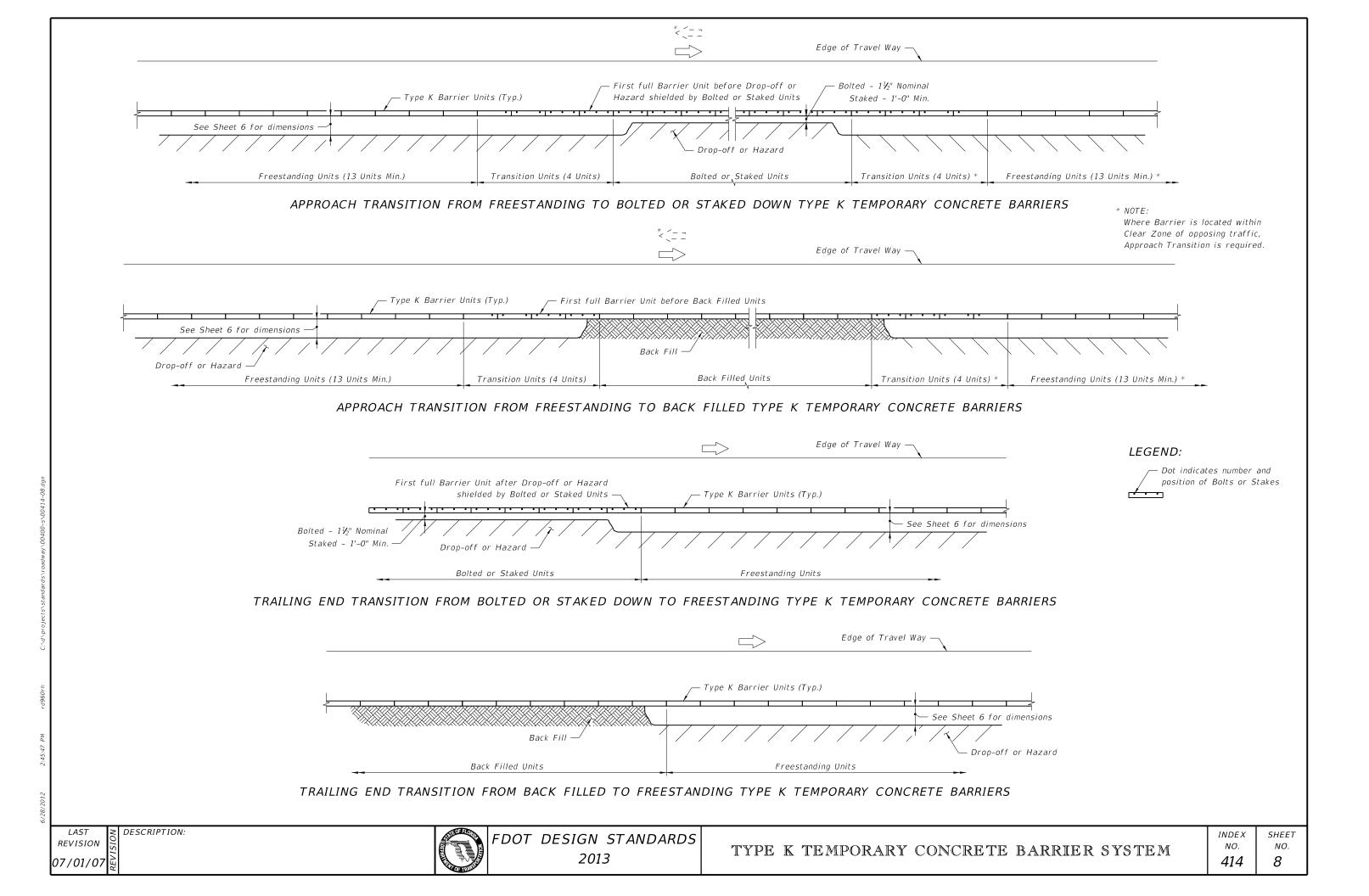
TYPE K TEMPORARY CONCRETE BARRIER SYSTEM

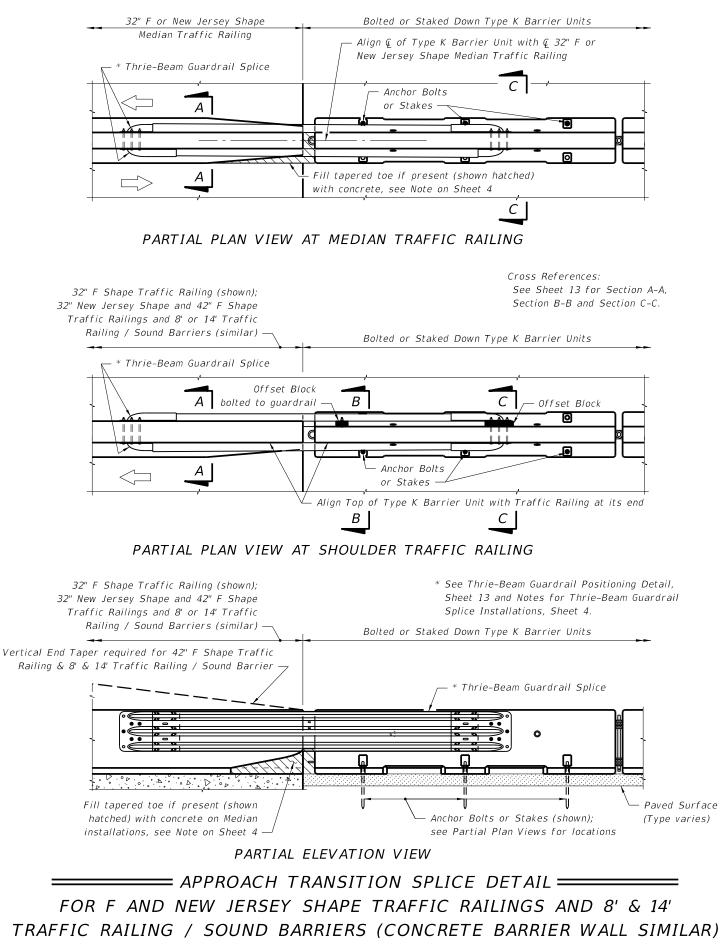
INDEX NO.

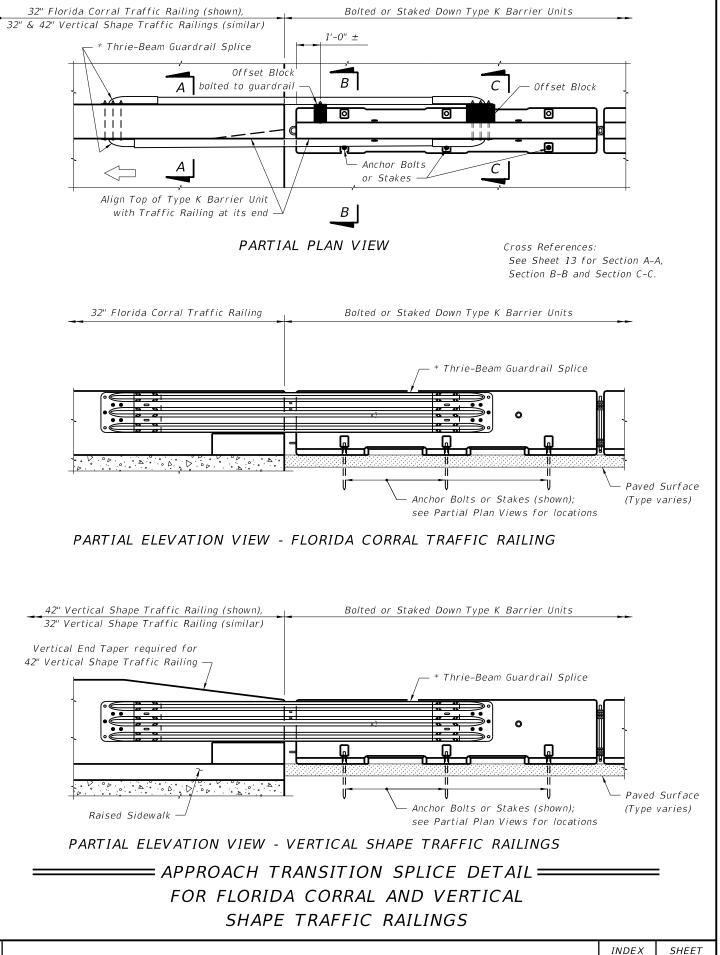
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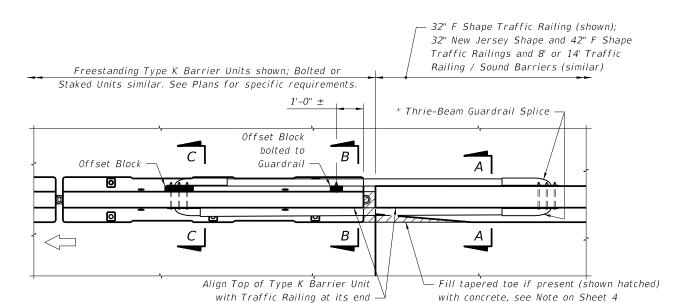


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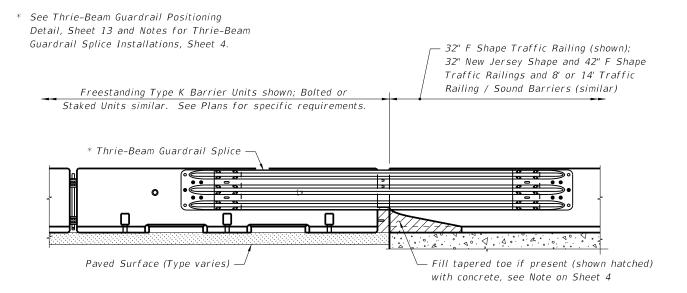
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07/01/01



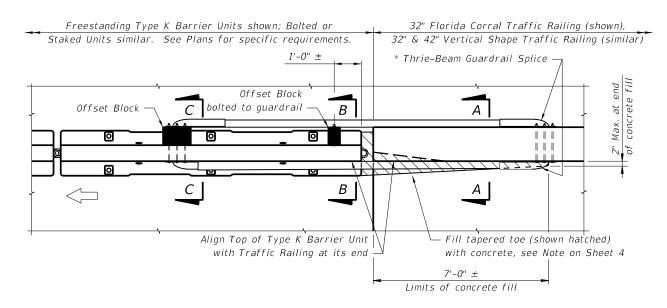
PARTIAL PLAN VIEW



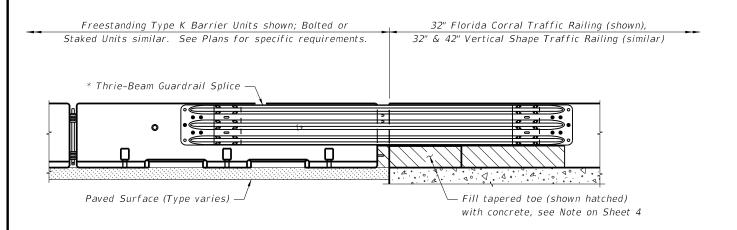
PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section A-A, Section B-B and Section C-C.

= TRAILING END SPLICE DETAIL == FOR F AND NEW JERSEY SHAPE TRAFFIC RAILINGS AND 8' & 14' TRAFFIC RAILING / SOUND BARRIERS



PARTIAL PLAN VIEW



PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section A-A, Section B-B and Section C-C.

= TRAILING END SPLICE DETAIL = FOR FLORIDA CORRAL AND VERTICAL SHAPE TRAFFIC RAILINGS

FDOT DESIGN STANDARDS 2013

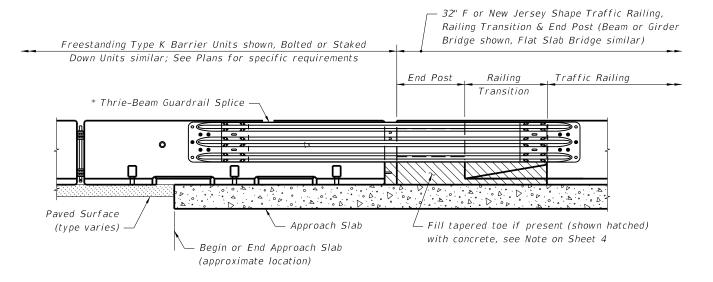
11

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DESCRIPTION:

PARTIAL PLAN VIEW

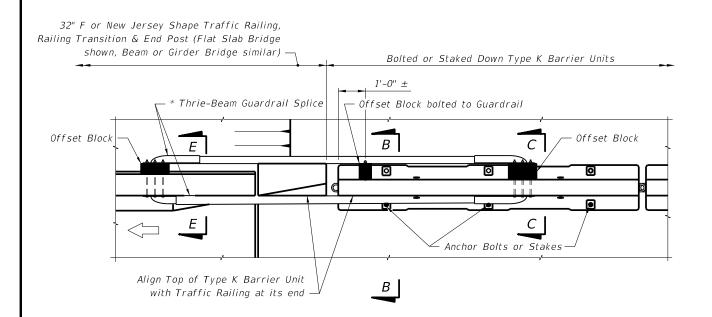
* See Thrie-Beam Guardrail Positioning Detail, Sheet 13 and Notes for Thrie-Beam Guardrail Splice Installations, Sheet 4.



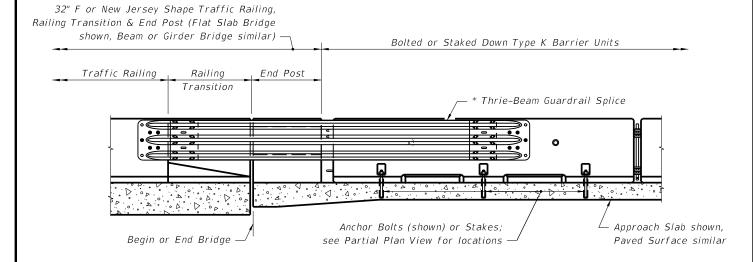
PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section B-B, Section C-C and Section D-D.

FOR 32" F AND NEW JERSEY SHAPE TRAFFIC RAILINGS
WITH RAILING TRANSITION AND END POST



PARTIAL PLAN VIEW



PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section B-B, Section C-C and Section E-E.

FOR 32" F AND NEW JERSEY SHAPE TRAFFIC RAILINGS
WITH RAILING TRANSITION AND END POST

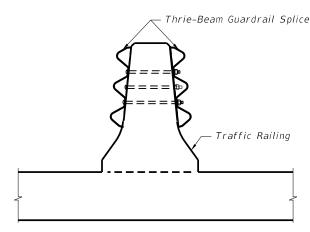
LAST REVISION 07/01/07

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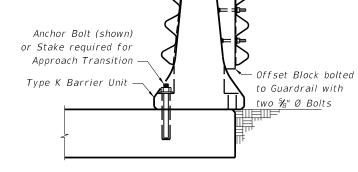
FDOT DESIGN STANDARDS
2013

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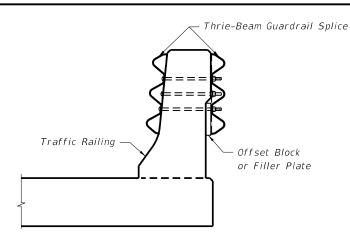


32" F Shape Median Traffic Railing (shown), Median Concrete Barrier Wall (similar)

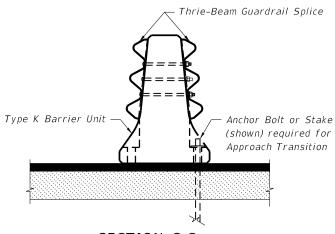




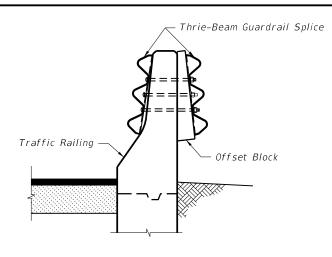
SECTION B-B Adjacent to Shoulder Traffic Railings



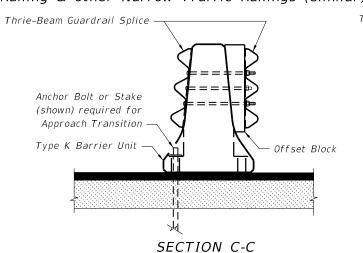
SECTION A-A 32" F Shape Traffic Railing (shown), 42" Traffic Railing and 8' & 14' Traffic Railing / Sound Barriers (similar)



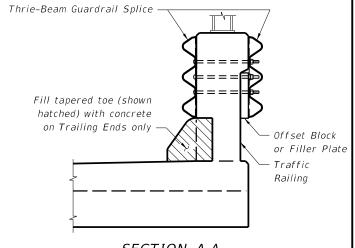
SECTION C-C Adjacent to 32" F or New Jersey Shape Median Traffic Railing or Median Concrete Barrier Wall



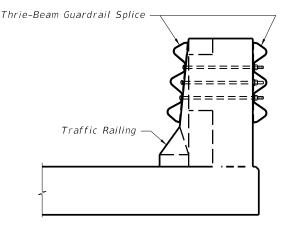
SECTION A-A 32" New Jersey Shape Concrete Barrier Wall (shown), 32" New Jersey Shape Traffic Railing & other Narrow Traffic Railings (similar)



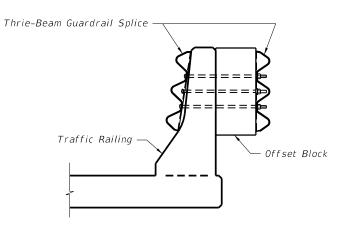
Adjacent to Shoulder Traffic Railings



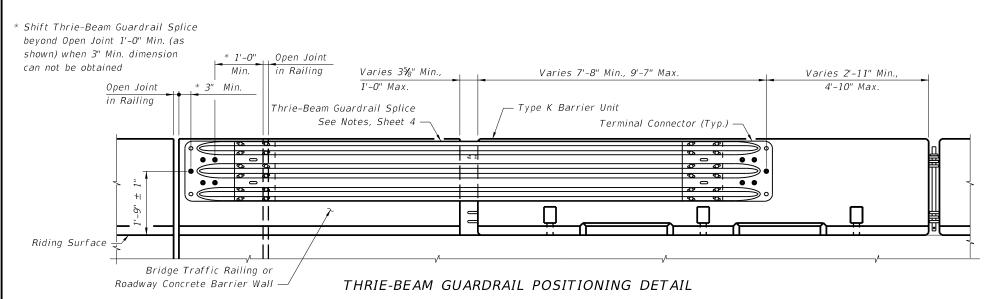
SECTION A-A 32" & 42" Vertical Shape Traffic Railing (shown), Florida Corral Traffic Railing (similar)



SECTION D-D 32" F or New Jersey Shape Traffic Railing, Railing Transition & End Post



SECTION E-E 32" New Jersey Shape Traffic Railing (shown), 32" F Shape Traffic Railing (similar)



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DESCRIPTION:

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DESCRIPTION:



MEDIAN APPROACH AND TRAILING END TRANSITIONS FROM FREESTANDING TYPE K TEMPORARY CONCRETE BARRIERS TO FREESTANDING PROPRIETARY TEMPORARY BARRIERS

TYPE K-PROPRIETARY TEMPORARY CONCRETE BARRIER TRANSITIONS

First full Type K Barrier Unit before Back Filled Units

DESCRIPTION: REVISION 07/01/07



Freestanding Proprietary Barrier Units

Freestanding Type K Barrier Units

— Type K-Proprietary Barrier Transition Unit A or B (See QPL) $-\!\!\!/$

- Edge of Travel Way

- Barrier Units (Typ.)

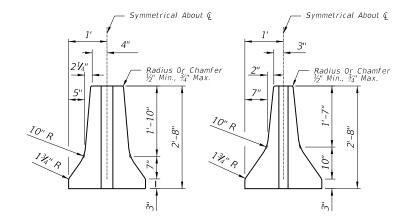
Freestanding Proprietary Barrier Units

Where Barrier is located within Clear Zone of opposing traffic,

Approach Transition is required.

- b. Proprietary temporary concrete barrier systems meeting NCHRP Report 350 Test Level 3 criteria which are included on the Qualified Products List.
- 2. Barrier units of dissimilar types may be interconnected within a single line barriers using transition units.
- 3. Alignment, length of need, anchorage and end treatment shall be in accordance with this Index.
- 4. Temporary concrete barrier units shown herein shall not be used for permanent barrier wall construction regardless of unit length.
- 5. If the plans specify Barrier (Temporary) (Type K), substitution with other barrier types is not permitted.
- 6. If the plans specify temporary concrete barrier system, substitution with water filled barriers is not permitted.
- 7. Where existing pavement is not present, construct an Asphalt Pad using Miscellaneous Asphalt Pavement. Cost of the Asphalt Pad to be included in the cost of the Barrier system.
- 8. Type C Steady-Burn Lights are to be mounted on top of temporary concrete barriers that are used as barriers along traveled ways in work zones. The lights are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways. For additional information refer to Index 600.
- 9. Barrier units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier (Temporary), LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights, Temporary, Barrier Mount (Steady-Burn), ED.
- 10. Deflection space shall be clear of any construction debris, stockpiled materials, equipment, and objects.

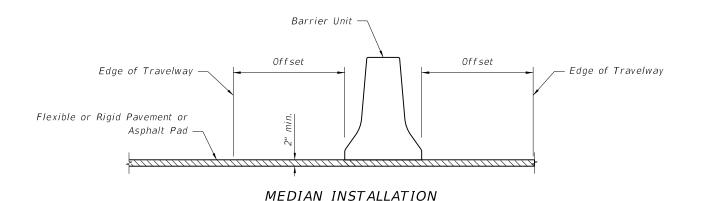
OFFSET AND DEFLECTION SPACE REQUIREMENTS					
Installation	Shielding	Work Zone Speed	Offset to Travelway	Deflection Space	
	Above Ground	45 mph or Less	1' min, 2' preferred	2' min.	
	Hazards	50 mph and Greater	2' min, 4' preferred	4' min.	
Right	Drop-Off	45 mph or Less	1' min, 2' preferred	2' min.	
Shoulder		50 mph and Greater			
	Hazards	a. Drop-offs 4' or Less and NO traffic below	2' min, 4' preferred	2' min.	
		b. All drop-off conditions other than 'a'	2' min, 4' preferred	4' min.	
Separating	g Adjacent Opposing	45 mph or Less	1' min, 2' preferred	1' min., 2' prefered	
Traffic	Traffic	50 mph and Greater	2' min, 4' preferred	2' min., 4' preferred	

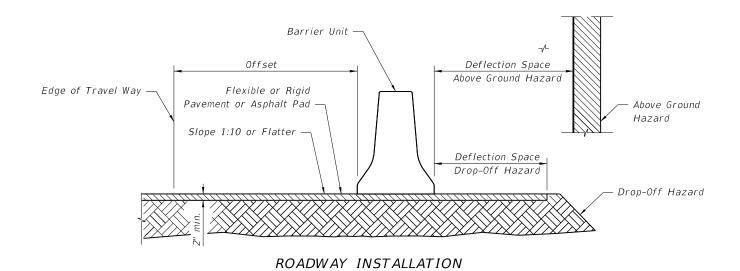


F-SHAPE

N.J. SHAPE

END VIEWS REINFORCEMENT AND OTHER UNIT FABRICATION DETAILS NOT SHOWN. PERMITTED BARRIER UNIT END VIEWS





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flare rates to be applied are 1:10 or flatter for speeds \leq 45 mph and 1:15 or flatter for speeds \geq 50 mph; see Index No. 642 for other flare rates on freeway facilities.

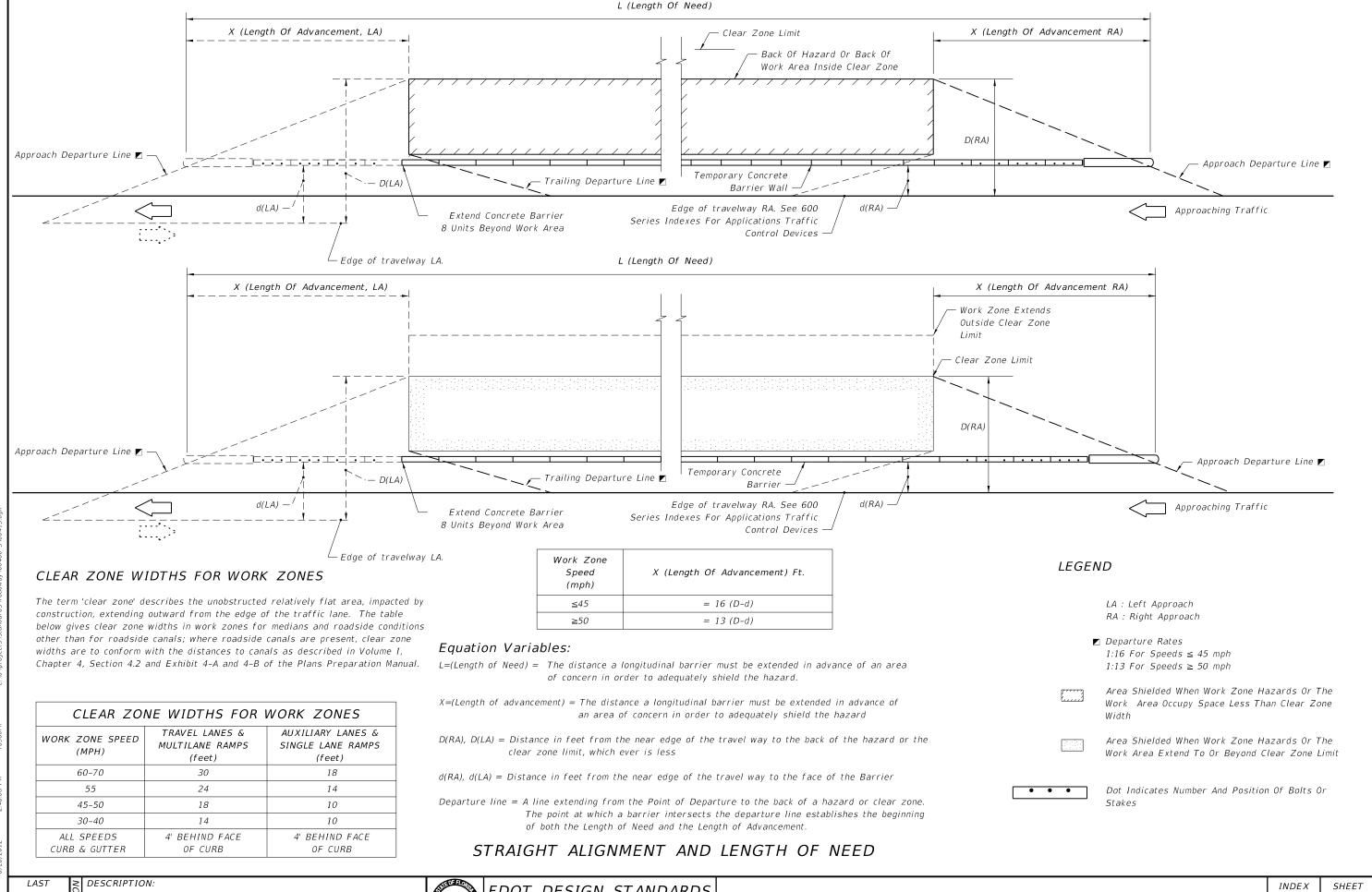
The surface cross slope approaching the barrier wall and continuing across the required deflection space shall not exceed a rate of 1 vertical: 10 horizontal.

FDOT DESIGN STANDARDS 2013

ALIGNMENT AND LENGTH OF NEED

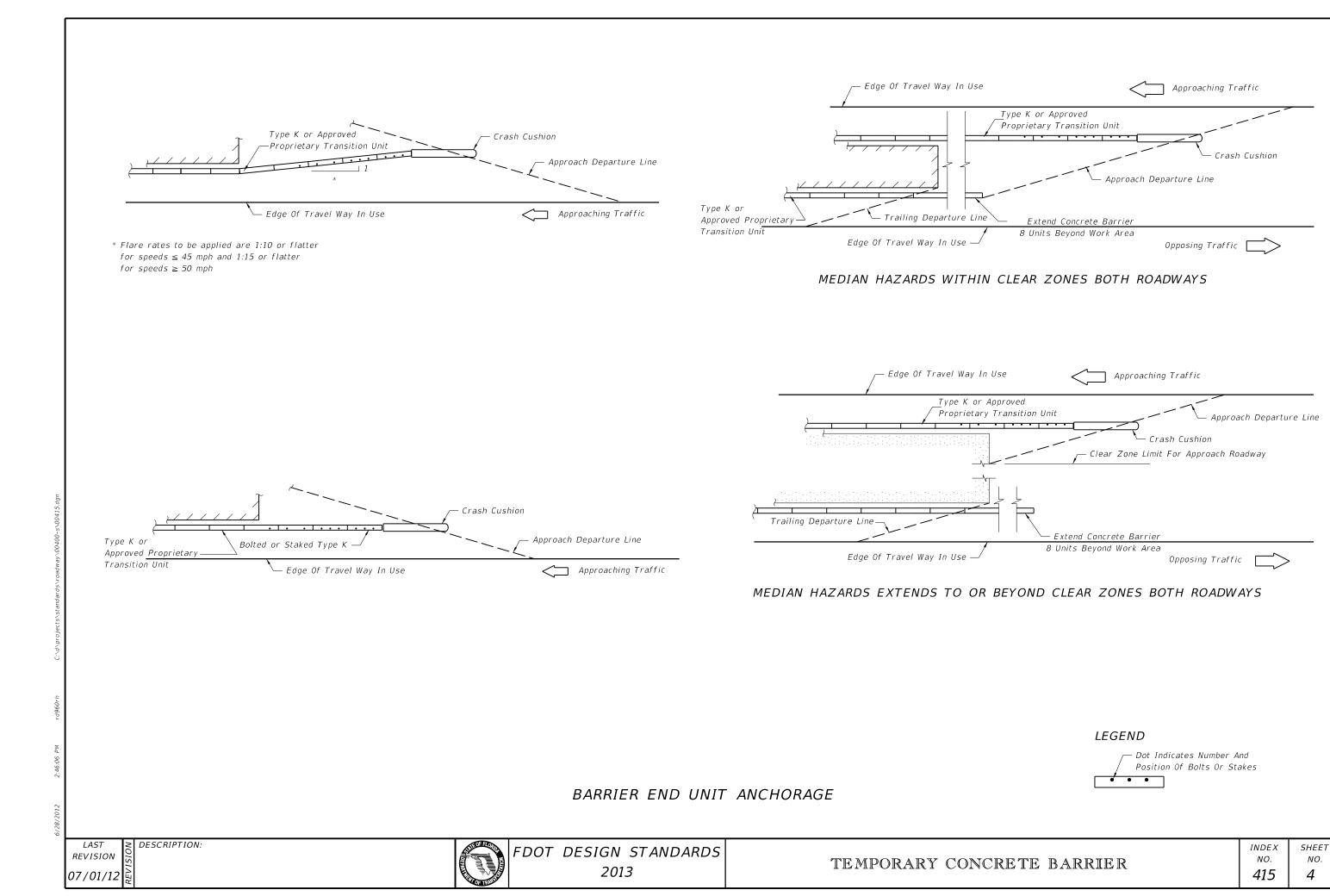
INDEX SHEET NO. NO. 415 2

BARRIER | 4



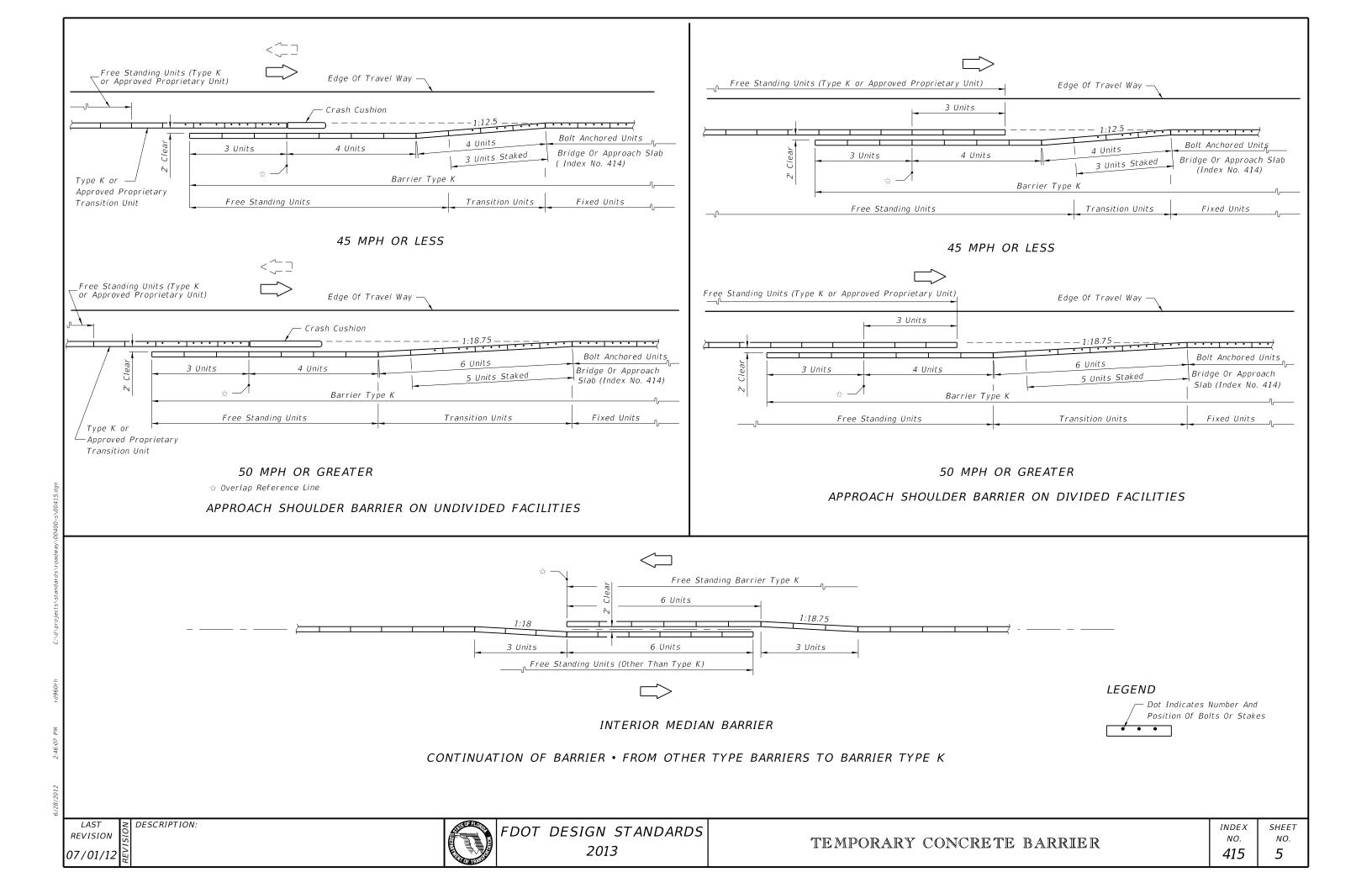
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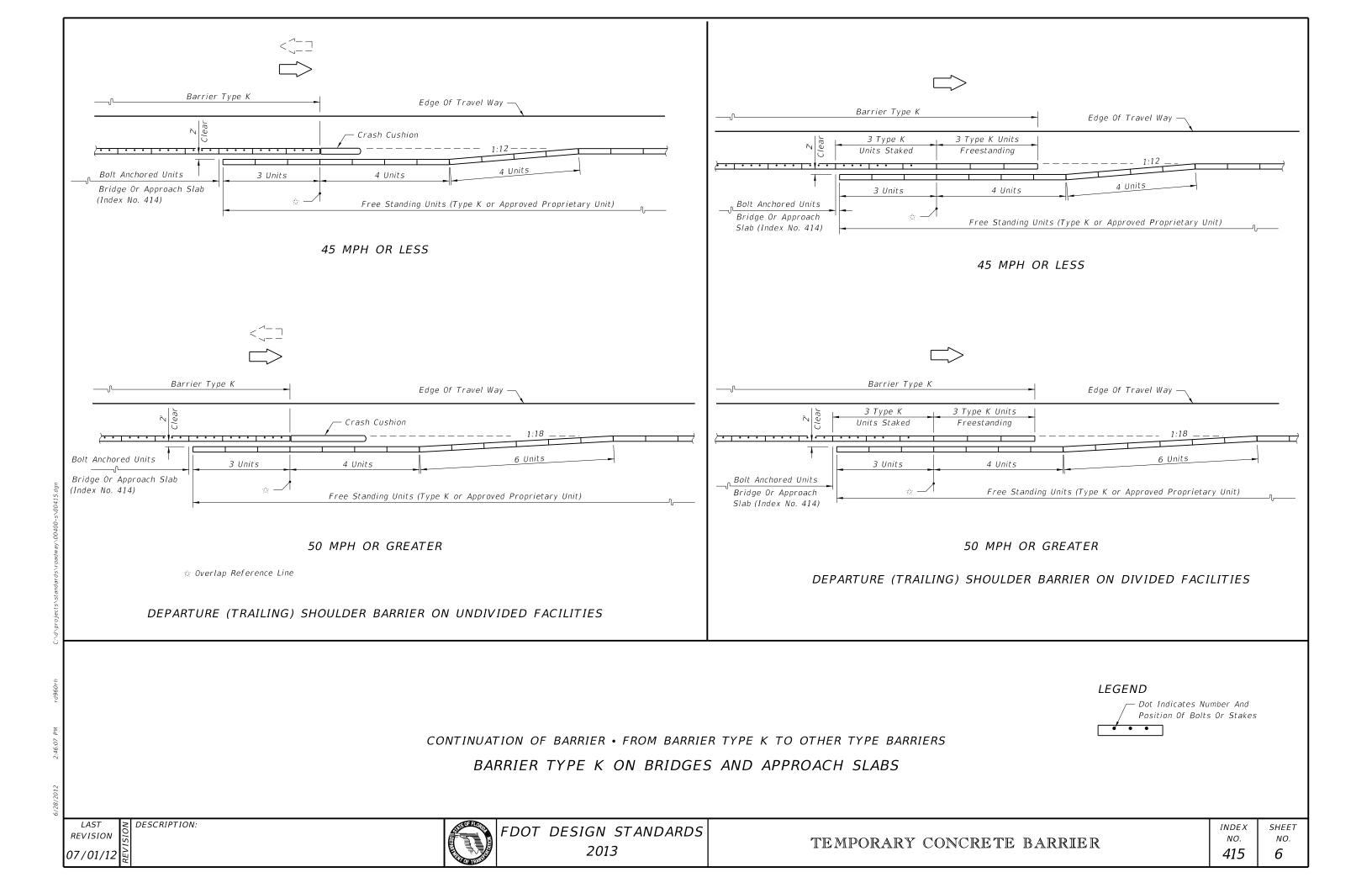
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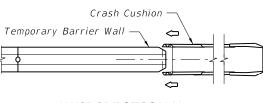


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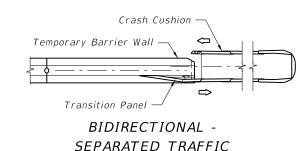
REVISION

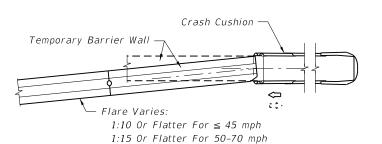
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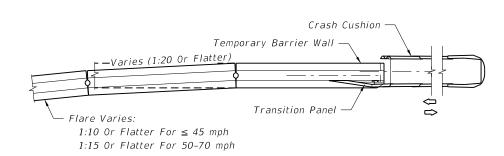








TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED OUTSIDE OPPOSING LANE CLEAR ZONE OR ONE-WAY TRAFFIC



TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED
WITHIN OPPOSING LANE CLEAR ZONE

LEGEND

Dot Indicates Number And

Position Of Bolts Or Stakes

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN) WALL END TREATMENT WHEN SHIELDED BY A CRASH CUSHION

NOTES FOR WALL END SHIELDING

- 1. Redirective crash cushions are the principal (standard) device to be used for shielding approach ends of temporary concrete barrier walls. The contractor has the option to construct any of the redirective crash cushions listed on the Qualified Products List at "102 Temporary Crash Cushion", subject to the uses and limitations described on their respective drawings. The barrier wall four end unit abutting crash cushions must be anchored to a paved surface using anchors/stakes in accordance with Standard Index 414.
- 2. Temporary redirective crash cushions shall be installed in accordance with the manufacturer's specifications and recommendations. Temporary crash cushions can be either new or functionally sound used devices. Performance of intended function is the only condition for acceptance, whether the crash cushion is new, used, refurbished, purchased, leased, rented, on loan, shared between projects, or made up of mixed new and used components.
- 3. A yellow post mounted Type 1 Object Marker shall be centered 3' in front of the nose of all temporary crash cushions. Mounting hardware shall be in accordance with Index Nos. 11860. The cost of the Object Marker shall be included in the cost of the crash cushion.
- 4. Equipment, stockpile material, etc., shall not be placed behind the crash cushion.
- 5. Optional temporary redirective crash cushions are to be paid for per location under the contract unit price for Crash Cushion (Redirective Option) (Temporary), LO.

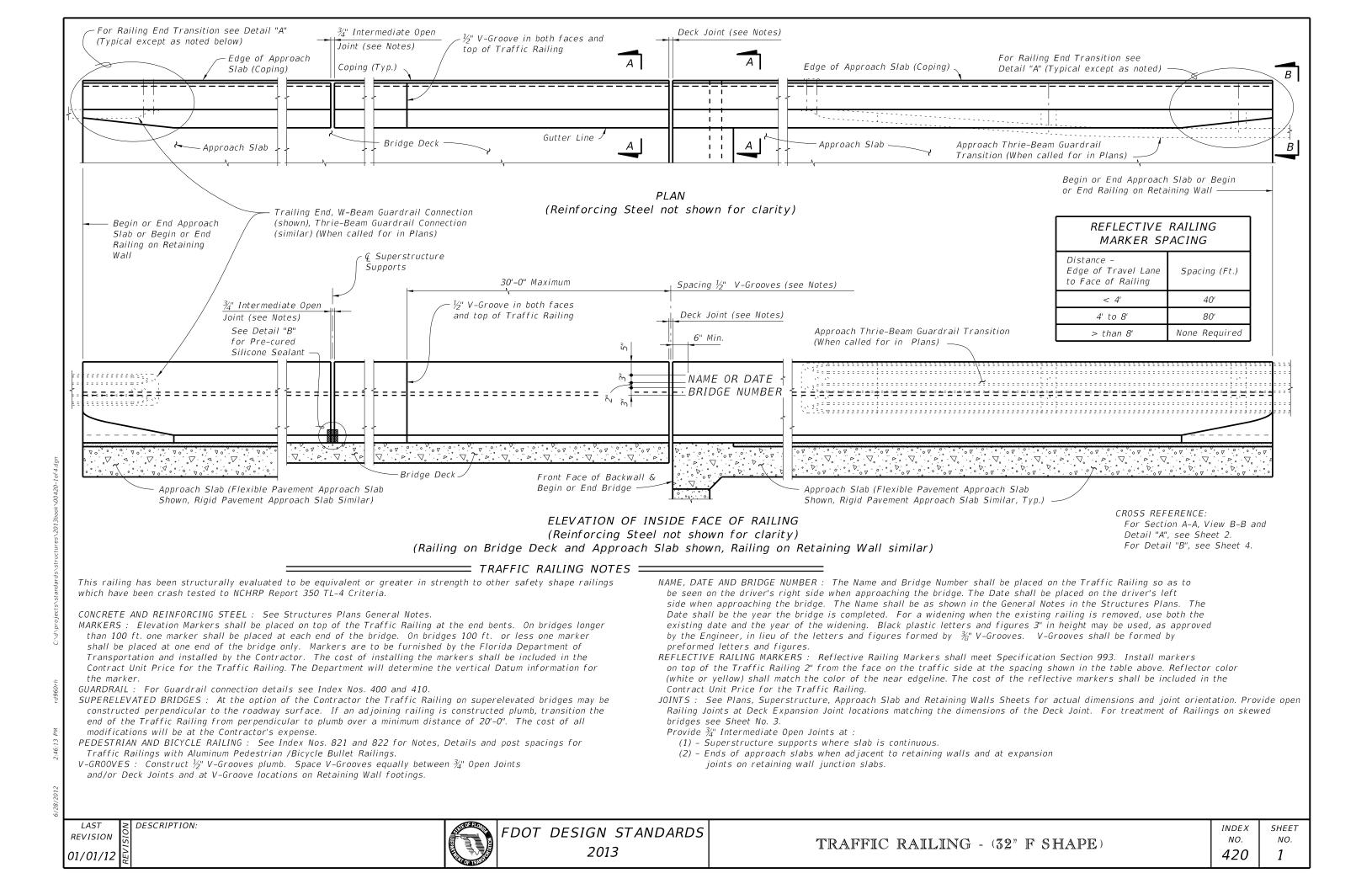
SHIELDING WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION)

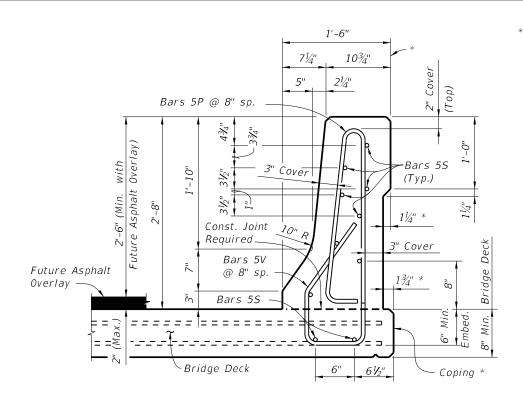
FDOT DESIGN STANDARDS
2013

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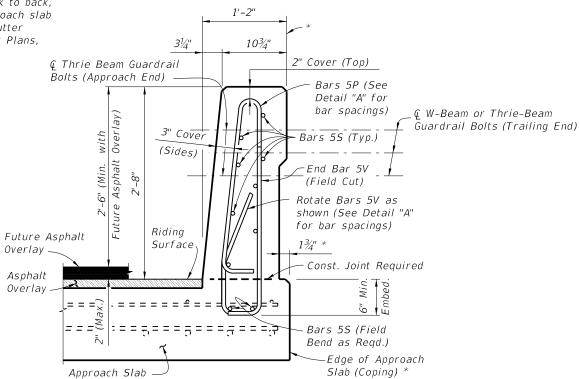
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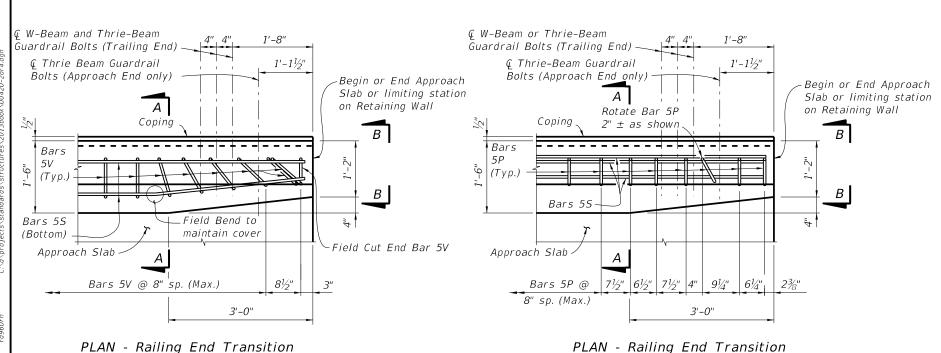


* Where railings of adjacent bridges are to be built back to back, the outside vertical plane of the railing and deck/approach slab may coincide along a plane centered 1'-6" from each gutter line. A bond breaker will be required. See Structures Plans, Superstructure Sheets for Details.



VIEW B-B (Section thru Approach Slab shown, Section thru Retaining Walls similar)

SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING (Section thru Bridge Deck shown, Section thru Approach Slab and Retaining Walls similar)



NOTES:

Rotate Bars 5V in Railing End Transition to maintain cover. Begin placing Railing Bars 5P and 5V on Approach Slab at the barrier end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.

CROSS REFERENCE:
For locations of Section A-A and
View B-B see Sheet 1.

______ DETAIL "A" _______ (Railing on Approach Slab shown, Railing on Retaining Wall similar)

NOTE: Omit Railing End Transition and Guardrail if Index 410 Concrete Barrier Wall is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5P and 5V at 8" (Typ.)

LAST REVISION O7/01/12

(Showing Bars 5V and 5S)



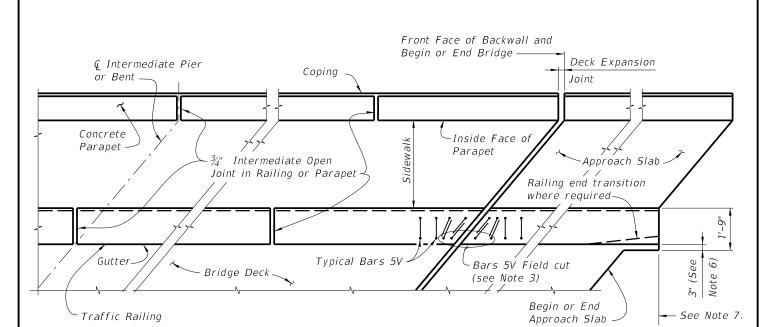
(Showing Bars 5P and 5S)

LAST

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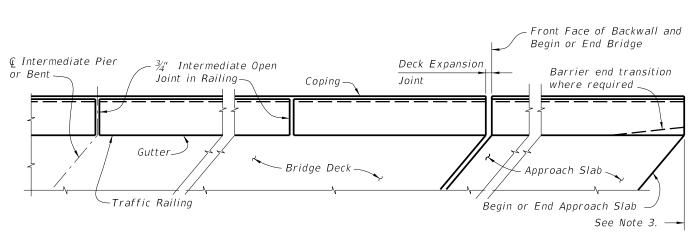
01/01/11

DESCRIPTION:



PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK, F SHAPE TRAFFIC RAILING AND PEDESTRIAN/BICYCLE RAILING INDEX NO. 820, 825 or 826, OTHER TRAFFIC RAILINGS SIMILAR

- 1) Concrete Parapet reinforcement is not effected by skew angle, see Index No. 820 for details.
- 2) Parapet expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 3) Traffic Railing reinforcement vertical Bars 5V & 5P may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement. Bars 5V adjacent to expansion joints shall be field adjusted to maintain clearance and spacing, extra Bars 5V will be required. Bars 5V bottom horizontal portion shall be cut so as to maintain maximum bottom horizontal length of bar to each vertical leg being placed, the remainder of bar shall be discarded. Cut Bars 5V may be rotated to maintain clearance.
- 4) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 5) ¾" Intermediate Open Joints and V-Grooves in railing and parapet shall be placed perpendicular or radial to the gutter line or inside face of parapet line. See Structures Plans, Superstructure Sheets for locations.
- 6) At begin or end approach slab extend slab at the railing ends 3" (gutter side or back face of railing as required) as shown to provide a base for casting of the railing.
- 7) Begin placing Railing Bars 5P and 5V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.



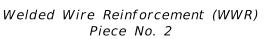
PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH F SHAPE TRAFFIC RAILING, OTHER TRAFFIC RAILINGS SIMILAR

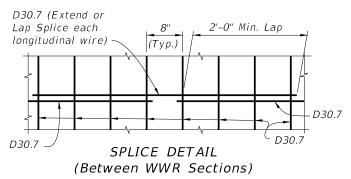
- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 2) $\frac{3}{4}$ " Intermediate Open Joints and $\frac{1}{2}$ " V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 3) Begin placing Railing Bars 5P and 5V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.

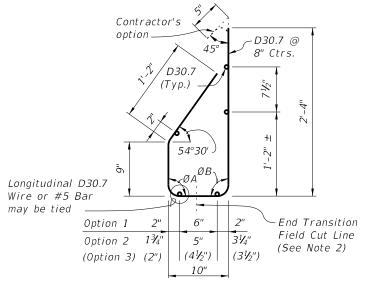
GENERAL NOTES:

- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes as applicable.
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at & Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing.
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

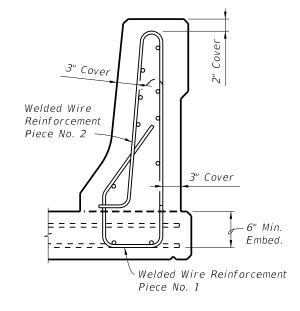








Welded Wire Reinforcement (WWR) Piece No. 1



WELDED WIRE REINFORCEMENT NOTES:

- 1. At the option of the Contractor Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 5P, 5S and 5V. Welded Wire Reinforcement shall conform to ASTM A497.
- 2. Welded Wire Reinforcement at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The vertical wires (D30.7) in Piece 1 shall be cut as shown and the gutter side portion bent inward as required to allow placement.

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

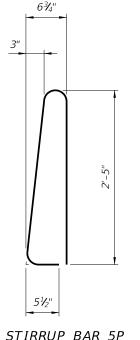
ROADWAY	LOW G	UTTER	HIGH (GUTTER
CROSS-SLOPE	ØA	ØB	ØA	ØB
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	9 <i>3</i> °
6% to 10%	96°	84°	84°	96°

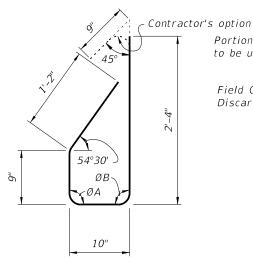
ØA and ØB shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.

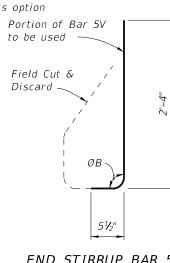
BILL OF REINFORCING STEEL			
MARK SIZE		LENGTH	
Р	5	5'-7"	
S	5	As Reqd.	
V	5	5'-1"	

Length as Required

BAR 5S





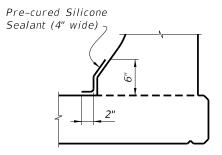


STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut (One Required per Railing End Transition)

REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 9" and the 2'-4" vertical dimensions shown for Bar 5V are based on a bridge deck without a raised sidewalk. If a raised sidewalk is to be provided, increase these dimensions to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above for a 8" deck with $\emptyset A = \emptyset B = 90^{\circ}$
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".

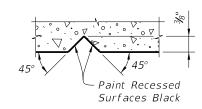


DESCRIPTION:

DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED QUA	TRAFFIC NTITIES	RAILING
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.104
Reinforcing Steel	LB/LF	27.12

(The above quantities are based on a 2% deck cross slope; railing on low side of deck.)

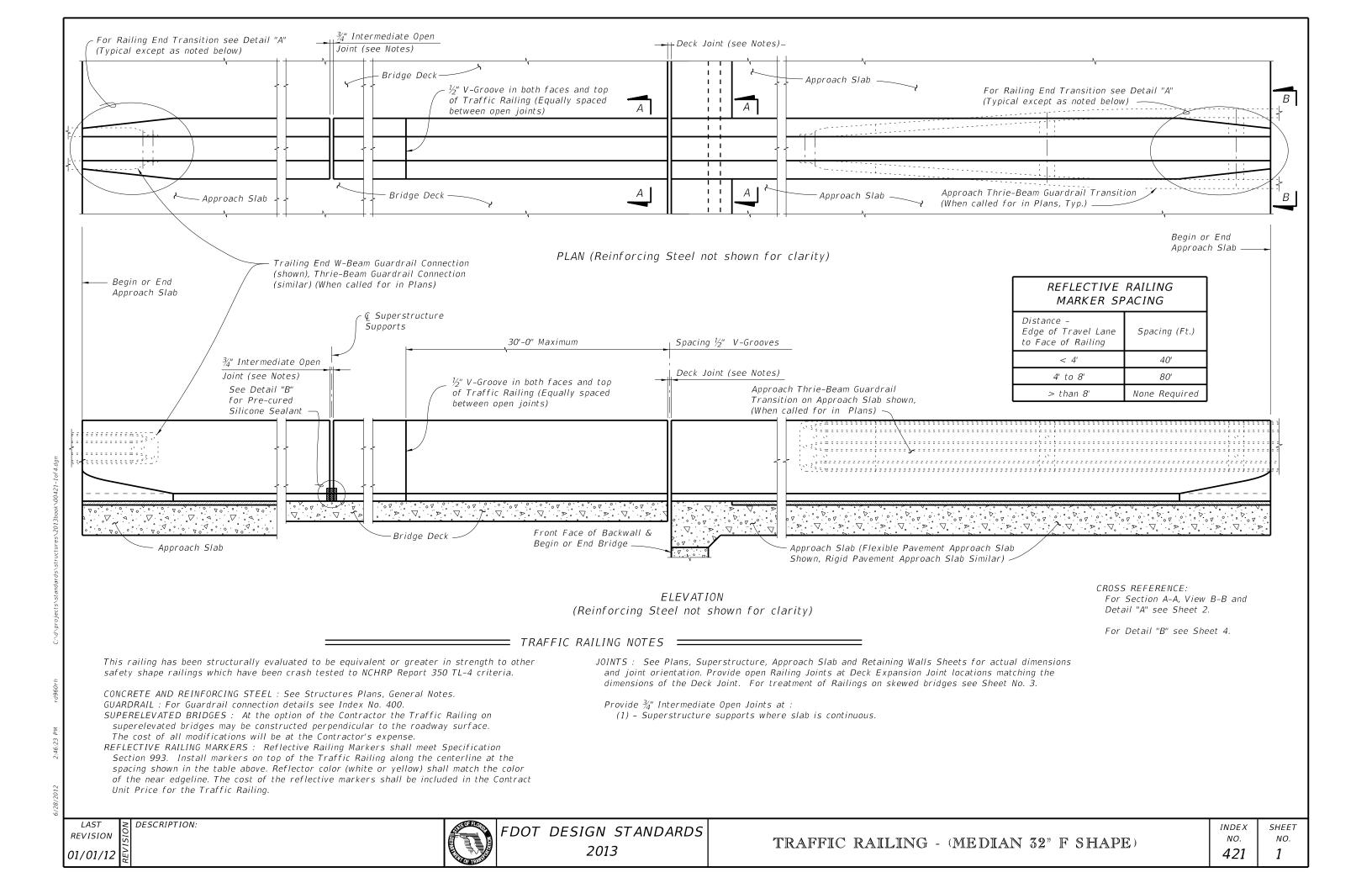
LAST REVISION 01/01/11

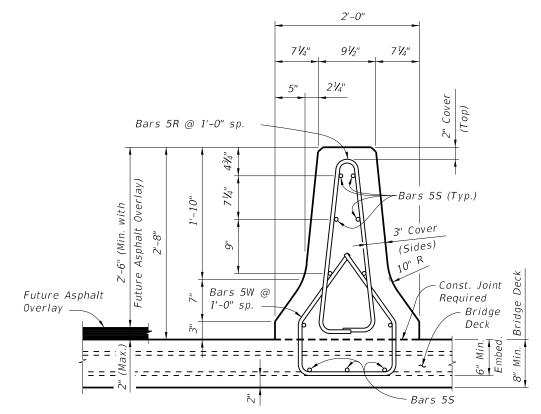


FDOT DESIGN STANDARDS 2013

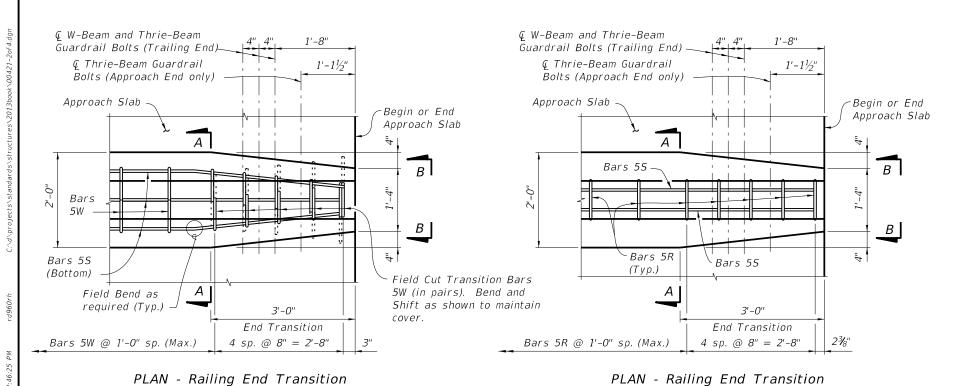
TRAFFIC RAILING - (32" F SHAPE)

INDEX NO. NO. 420 4





SECTION A-A
TYPICAL SECTION THRU TRAFFIC RAILING
(SECTION THRU BRIDGE DECK SHOWN SECTION THRU APPROACH SLAB SIMILAR)



1'-4" 31/4" 91/2" Bars 5R (See Detail "A" for bar spacings) ← Thrie-Beam Guardrail Bolts -Ç W-Beam or Thrie-Beam Guardrail Bolts (Trailing End) 3" Cover —Bars 5S (Typ.)(Sides) Field Cut, Shift and Bend Transition Future Bars 5W as shown to maintain cover Asphalt (See Detail "A" for bar spacings) Overlay Asphalt Overlay 5 -Const. Joint Required Riding Surface Min. Approach Slab Bars 5S

VIEW B-B

NOTE:

Begin placing Railing Bars 5R and 5W on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5R and 5W shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5R and 5W as required to maintain cover in Railing End Transition.

Omit Railing End Transition and Guardrail if Index 410 Concrete Barrier Wall is used beyond the Approach Slab. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Transition is omitted, extend Typical Section to the end of Approach Slab and space Bars 5R and 5W at 1'-0" (Typ.)

LAST REVISION 01/01/11

DESCRIPTION:

(Showing Bars 5W and 5S)



(Showing Bars 5R and 5S)

DETAIL "A"

FDOT DESIGN STANDARDS 2013

421

PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4) ¾" Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the Q of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.

FDOT DESIGN STANDARDS

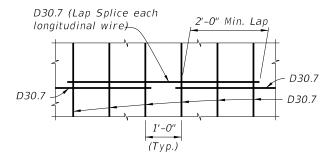
2013

- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing deformed bars with an equivalent area of steel.

LAST

DESCRIPTION:

Welded Wire Reinforcement (WWR) Piece No. 2

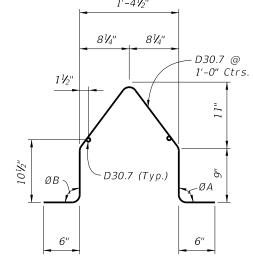


SPLICE DETAIL (Between WWR Sections)

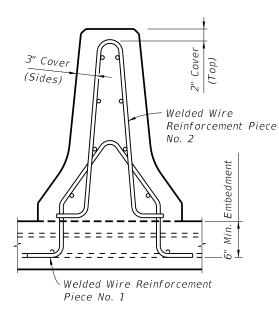
WELDED WIRE REINFORCEMENT NOTES:

DESCRIPTION:

- 1. At the option of the Contractor Welded Wire Reinforcement may be utilized in lieu of all Bars 5R, 5S and 5W. Welded Wire Reinforcement shall conform to ASTM A497.
- 2. Welded Wire Reinforcement at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The top of Piece 1 shall be cut to allow overlap.
- 3. Place WWR panels so as to minimize the end overhang of longitudinal wires at Railing Ends and Open Joints. Overhangs greater than 6" are not permitted.



Welded Wire Reinforcement (WWR) Piece No. 1



CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

Contractor's option

ROADWAY	ON SLOPE		AT CROWN	
CROSS-SLOPE	ØA	ØB	ØA	ØB
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	90°	90°
6% to 10%	96°	84°	90°	90°

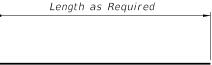
ØA and ØB shall be 90° if Contractor elects to place railing perpendicular to the deck, and approach slabs.

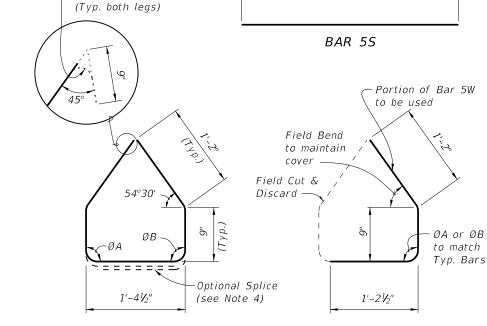
9¾"

3¾"

3"

BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
R	5	6'-1"		
S	5	As Reqd.		
W	5	5'-3''		





STIRRUP BAR 5R

STIRRUP BAR 5W

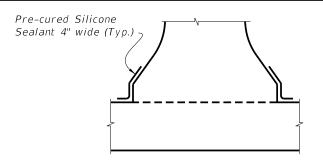
TRANSITION STIRRUP BAR 5W To Be Field Cut and Bent (10 required per Railing End Transition)

REINFORCING STEEL NOTES:

5½"

(Typ.)

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 4. At the Contractor's option, Bars 5W may be fabricated as a two piece bar with a 1'-2" lap splice of the bottom legs.



DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

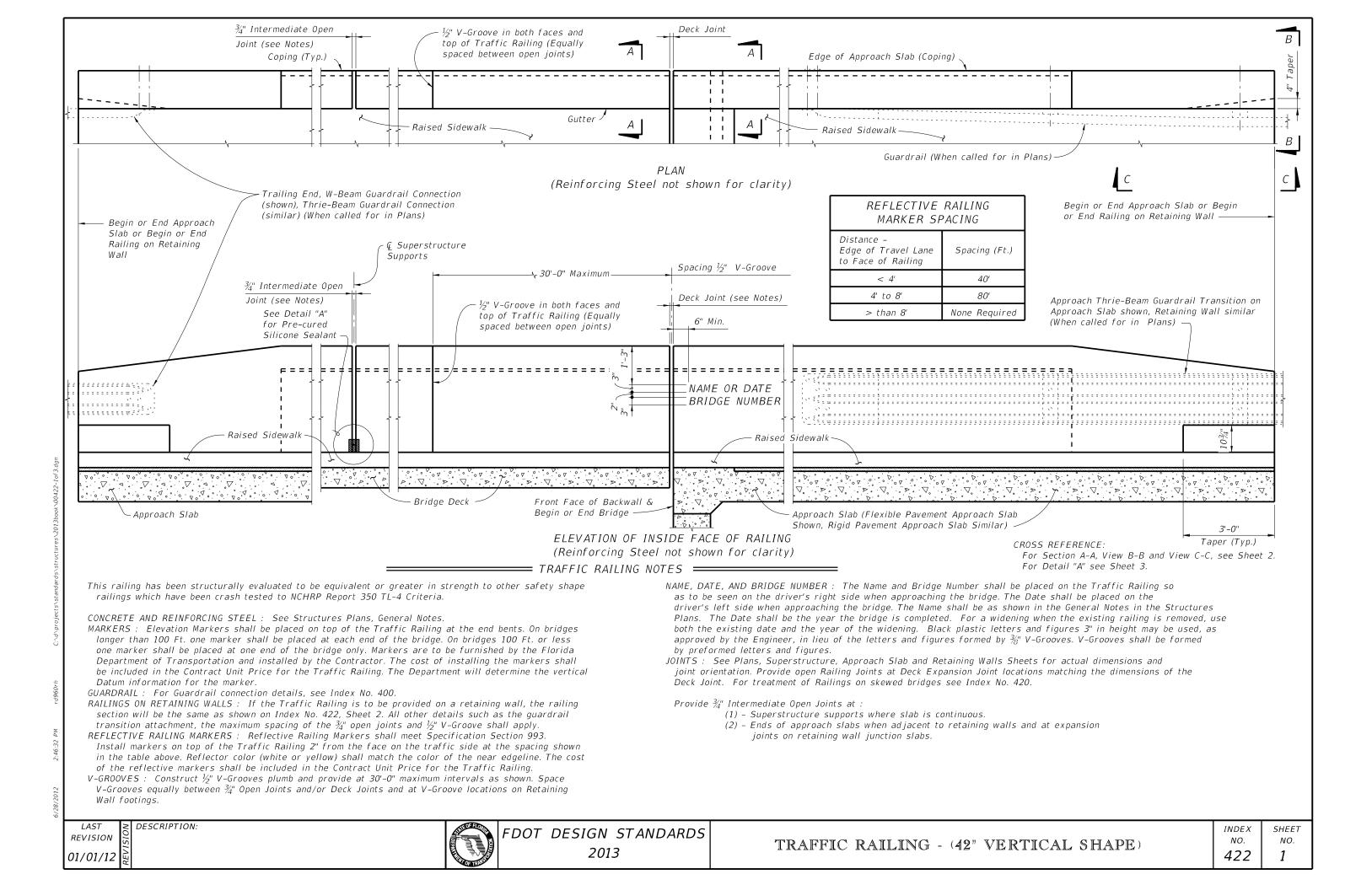
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM UNIT QUANTITY				
Concrete	CY/LF	0.120		
Reinforcing Steel	LB/LF	23.29		

(The above quantities are based on a crowned roadway, with a 2% cross slope)

LAST REVISION 01/01/11



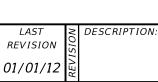


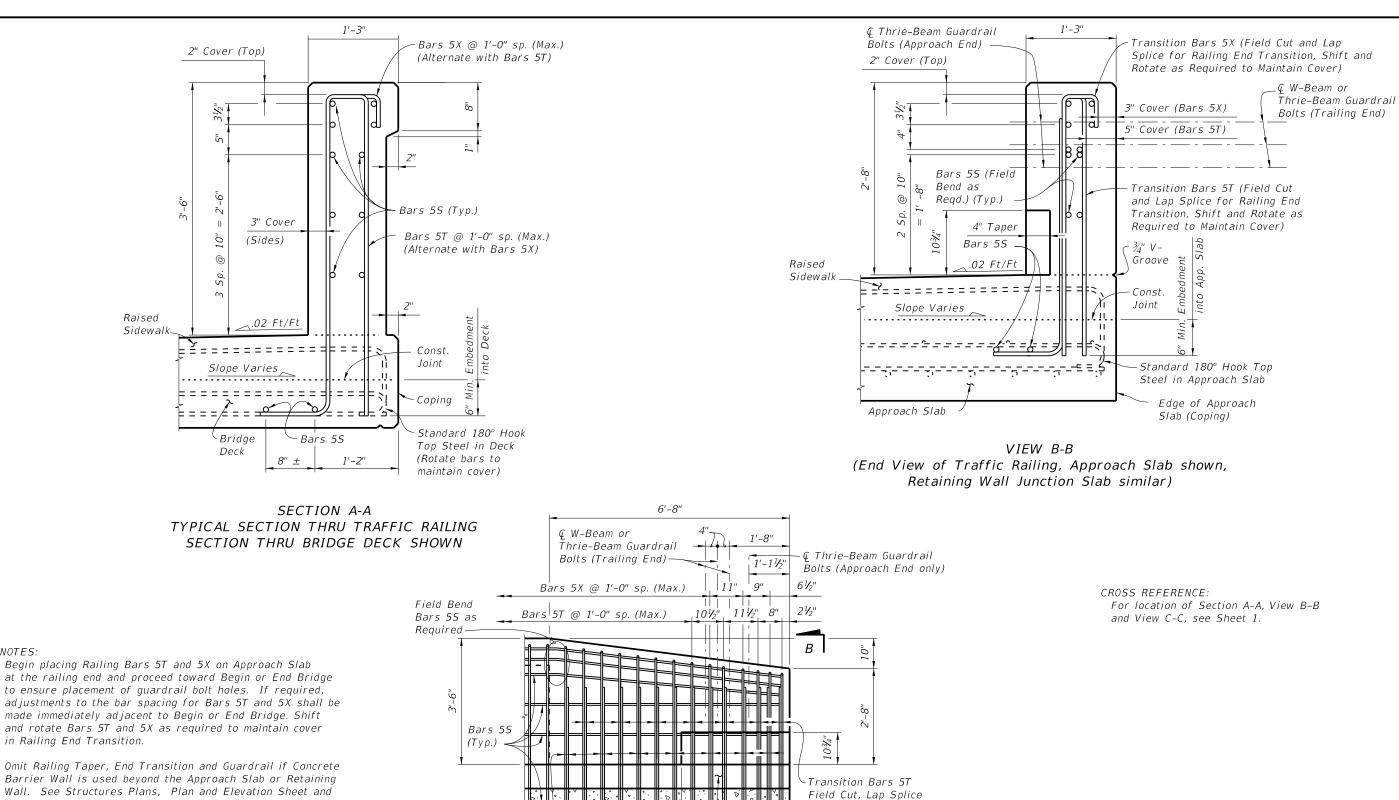


NOTES:









Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0" (Typ.)

> VIEW C-C RAILING END TRANSITION (Guardrail Not Shown For Clarity)

3'-0" Taper

Raised Sidewalk-

(2'-2" Min.)

Approach Slab

FDOT DESIGN STANDARDS 2013

Transition Bars 5X

Splice (2'-2" Min.) —

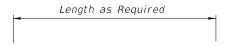
Field Cut & Lap

TRAFFIC RAILING - (42" VERTICAL SHAPE)

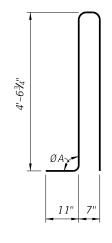
INDEX SHEET NO. NO.

BILL OF REINFORCING STEEL				
MARK SIZE		LENGTH		
S	5	As Reqd.		
Τ	5	10'-8"		
Χ	5	6'-9"		

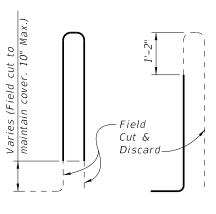
ROADWAY	ØA		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	83°	
6% to 10%	84°	96°	

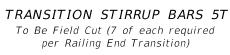


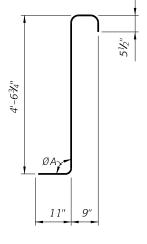
BAR 5S



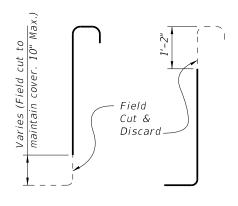
STIRRUP BAR 5T







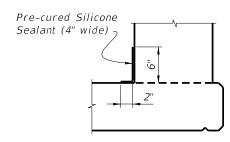
STIRRUP BAR 5X



TRANSITION STIRRUP BARS 5X To Be Field Cut (7 of each required per Railing End Transition)

REINFORCING STEEL NOTES:

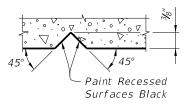
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 4'-6 $\frac{3}{4}$ " vertical dimension shown for Bars 5T and 5X is based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slope vary from the above amounts, adjust this dimension accordingly to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above with $\emptyset A = 90^{\circ}$.
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM UNIT QUANTITY				
Concrete	CY/LF	0.145		
Reinforcing Steel	LB/LF	30.68		

(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)

TRAFFIC RAILING - (42" VERTICAL SHAPE)

LAST REVISION 01/01/11

DESCRIPTION:

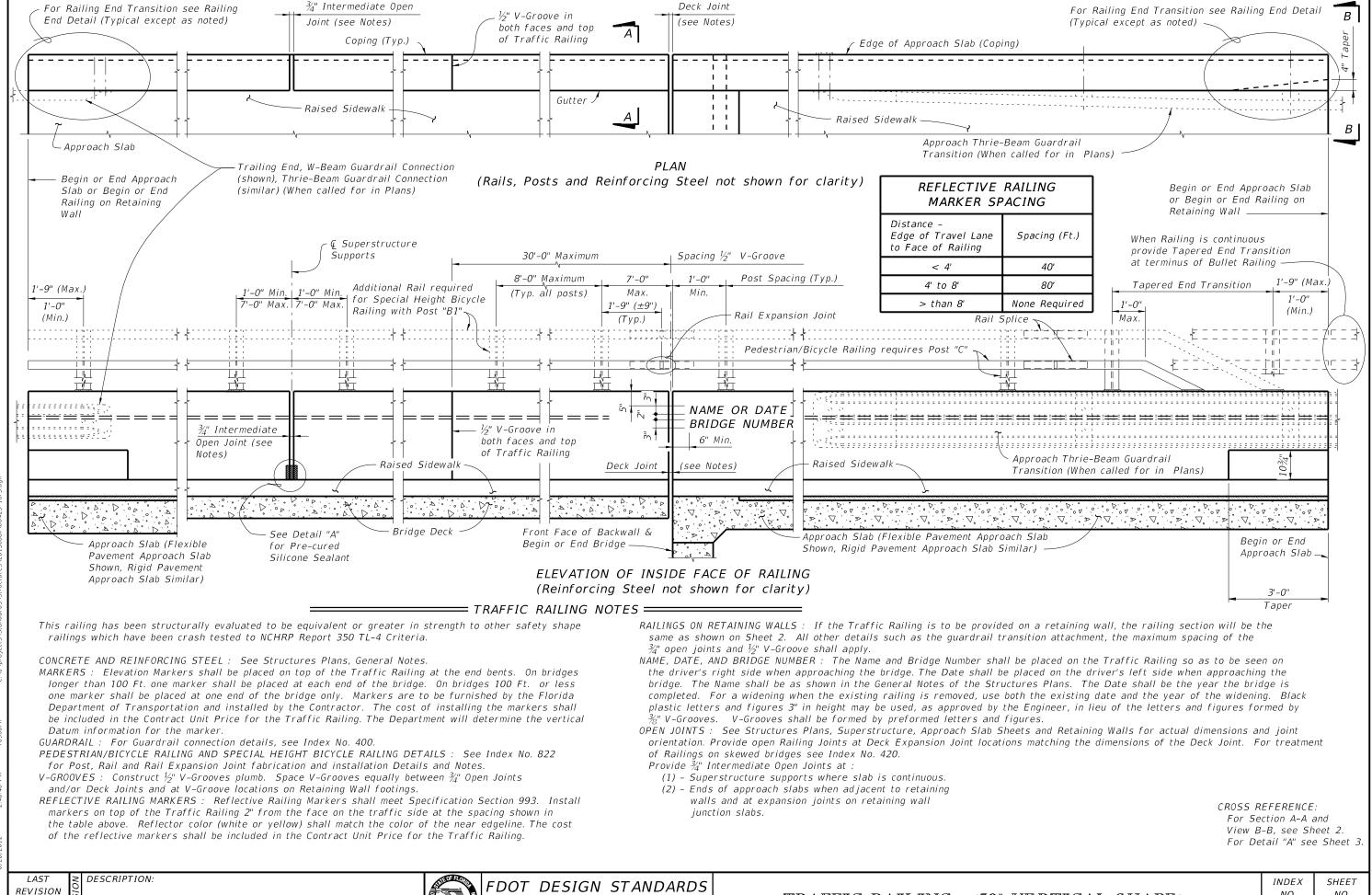


INDEX

SHEET

NO.

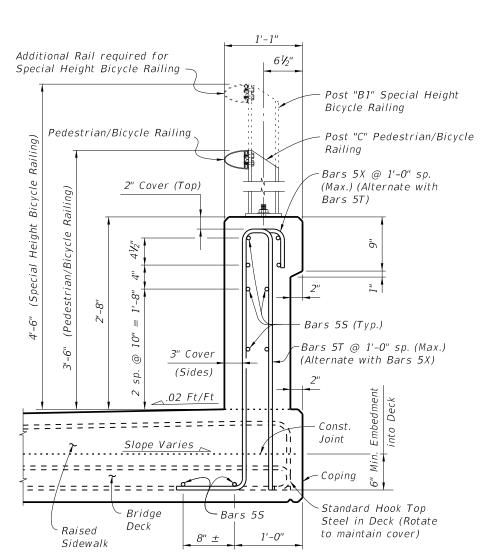
3



2013

01/01/12

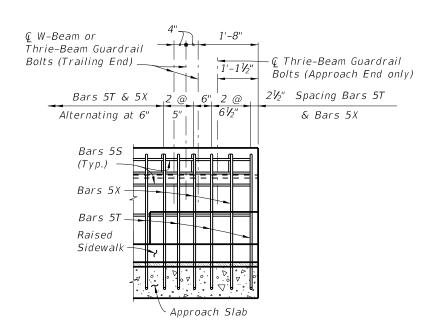




SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING SECTION THRU BRIDGE DECK SHOWN

DESCRIPTION:

Omit Railing End Taper and Guardrail if Concrete Barrier Wall is used beyond the Approach Slab. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Taper is omitted, extend Typical Section to the end of the Approach Slab. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5T and 5X on Approach Slab in end taper section as required to maintain cover.



APPROACH SLAB END VIEW OF TRAFFIC RAILING

VIEW B-B

8" ±

1'-1"

6½"

Post "B1" Special Height

Post "C" Pedestrian/Bicycle

Thrie-Beam Guardrail

Bolts (Trailing End)

Ç Thrie−Beam Guardrail Bolts (Approach End)

Bicycle Railing

-Bars 5X @ 1'-0" sp.

-Bars 5S (Field Bend as

-Bars 5T @ 1'-0" sp. (Max.)

Standard Hook Top

to maintain cover)

Edge of Approach Slab (Coping)

Steel in Deck (Rotate

(Alternate with Bars 5X)

Required) (Typ.)

Const.

Joint

Bars 5S

1'-0"

(Max.) (Alternate with

Railing

Bars 5T)

Additional Rail required for

Bicycle

ian/Bicycle

Special Height Bicycle Railing

Pedestrian/Bicycle Railing .

@

<u>|</u>

 γ Slope Varies $_{\sim}$

Raised

Sidewalk

- Bridge

Deck

2" Cover (Top)

 ← Thrie-Beam

4" Taper

.02 Ft/Ft

Guardrail Bolts

CROSS REFERENCE: For location of Section A-A and View B-B see Sheet 1.

NOTE: For Post "B1", Post "C" and Rail Details, see Index No. 822.

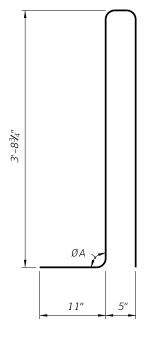
RAILING END DETAIL

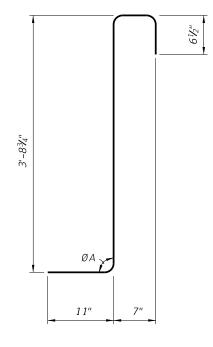
FDOT DESIGN STANDARDS 2013

REVISION 01/01/11

BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
S	5	As Reqd.		
Т	5	9'-0"		
Χ	5	5'-10"		

ROADW AY	Ø	A	
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	93°	
6% to 10%	84°	96°	





Length as Required

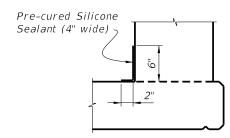
BAR 5S

STIRRUP BAR 5T

STIRRUP BAR 5X

REINFORCING STEEL NOTES:

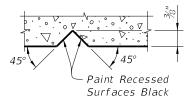
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 3'-8¾" vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into the bridge deck.
- 3. The reinforcement for the railing on a Retaining Wall shall be the same as detailed with $\emptyset A=90^\circ.$
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

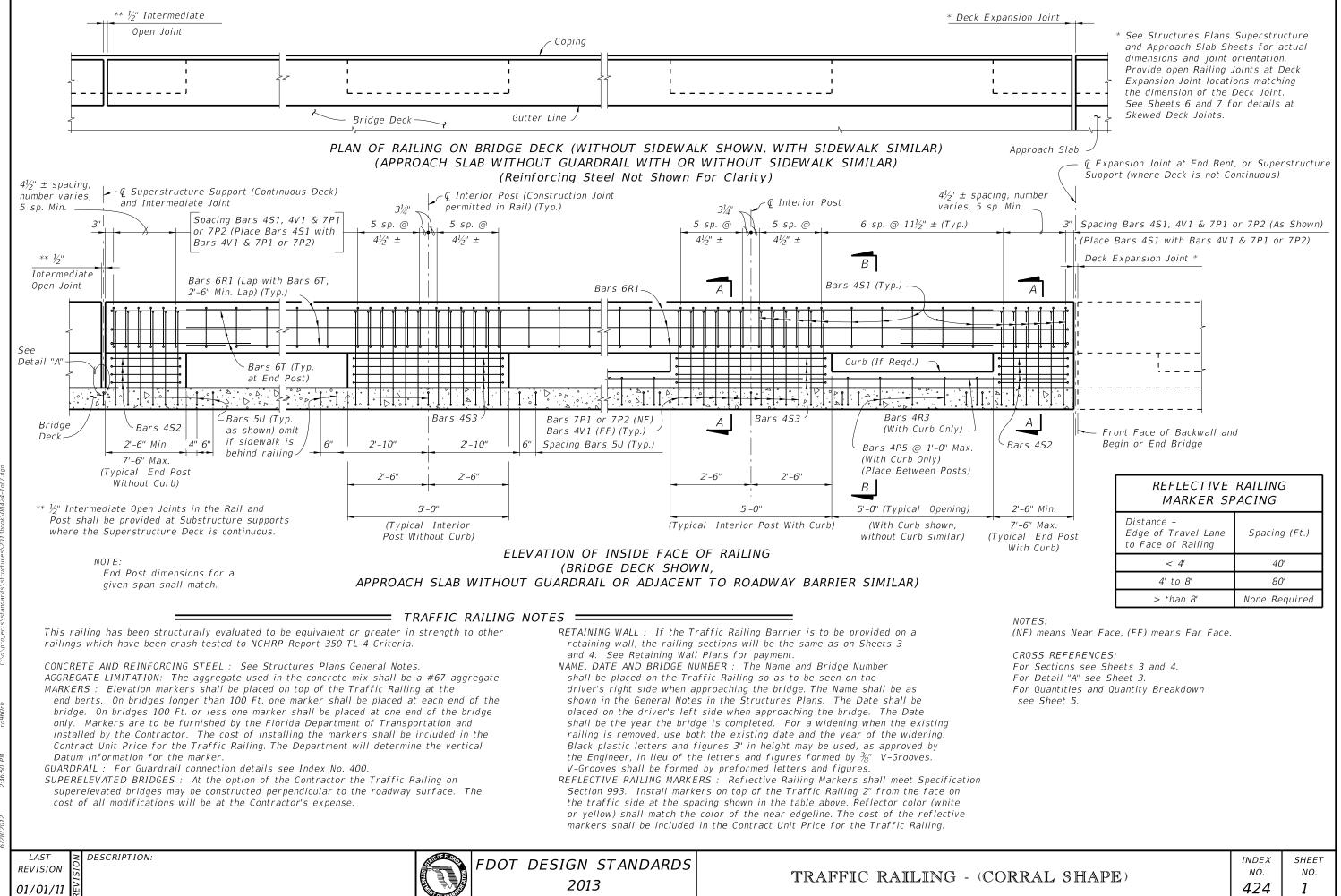
ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.095	
Reinforcing Steel	LB/LF	25.90	

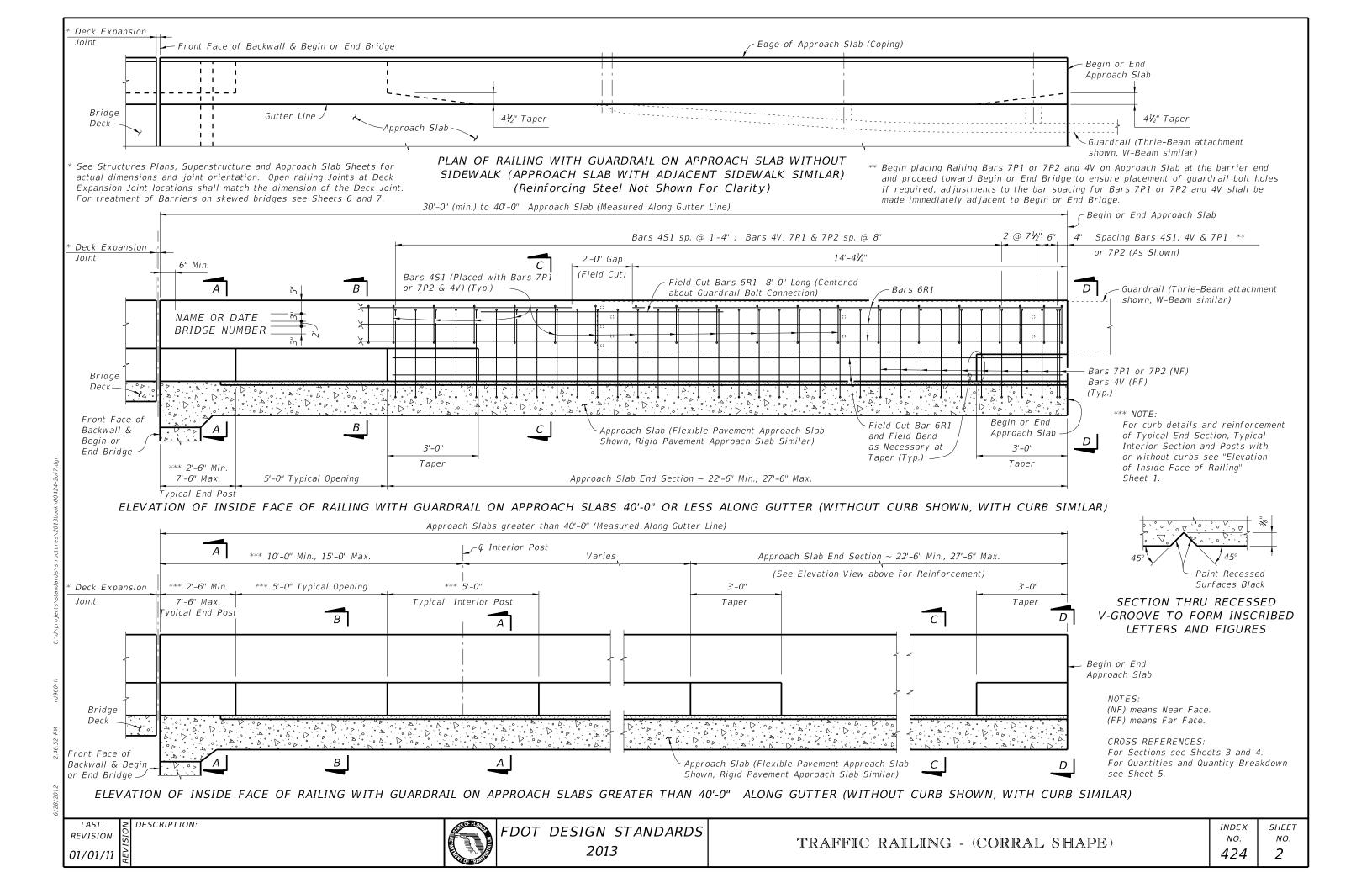
(The above quantities are based on a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope.)

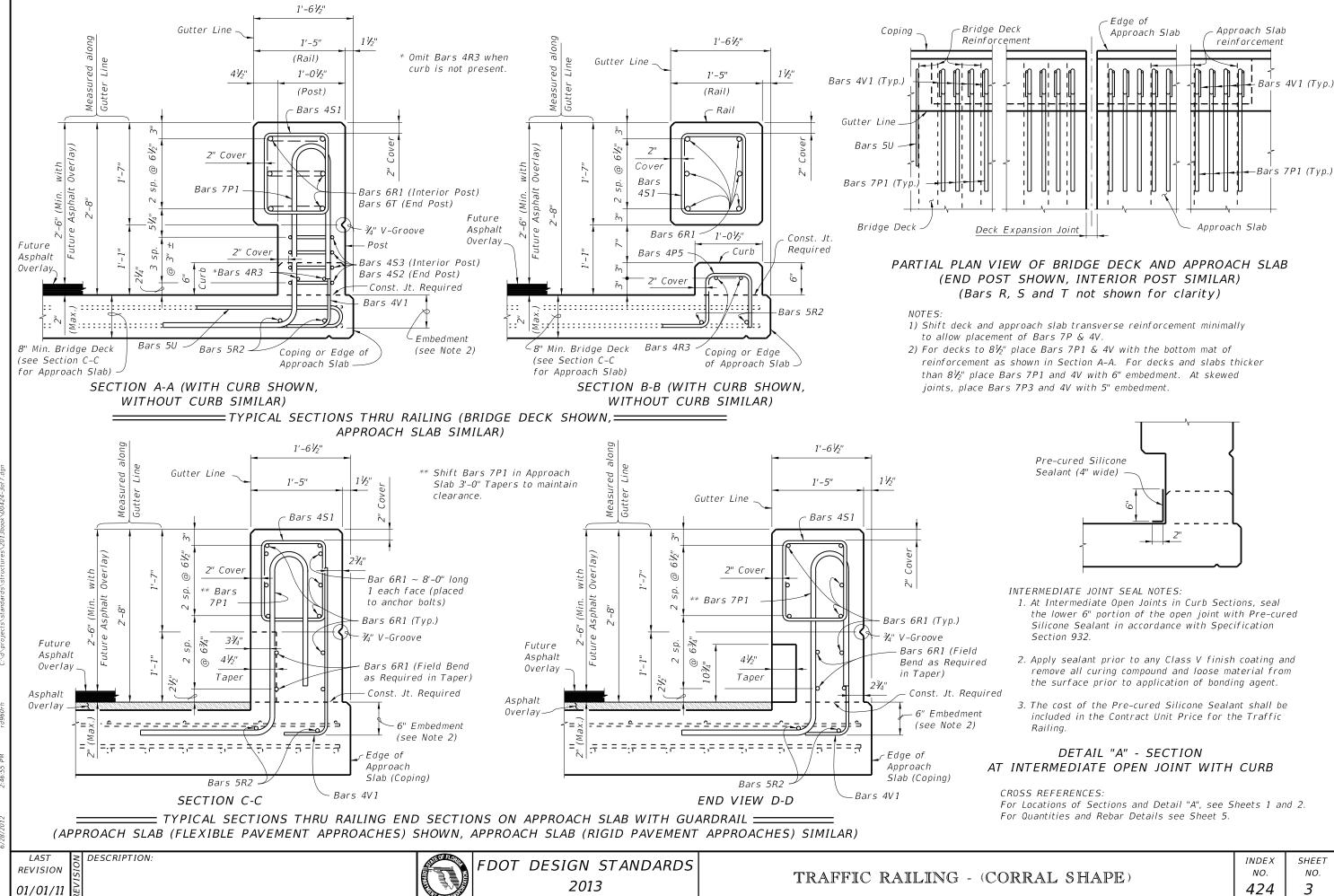
6/28/2012

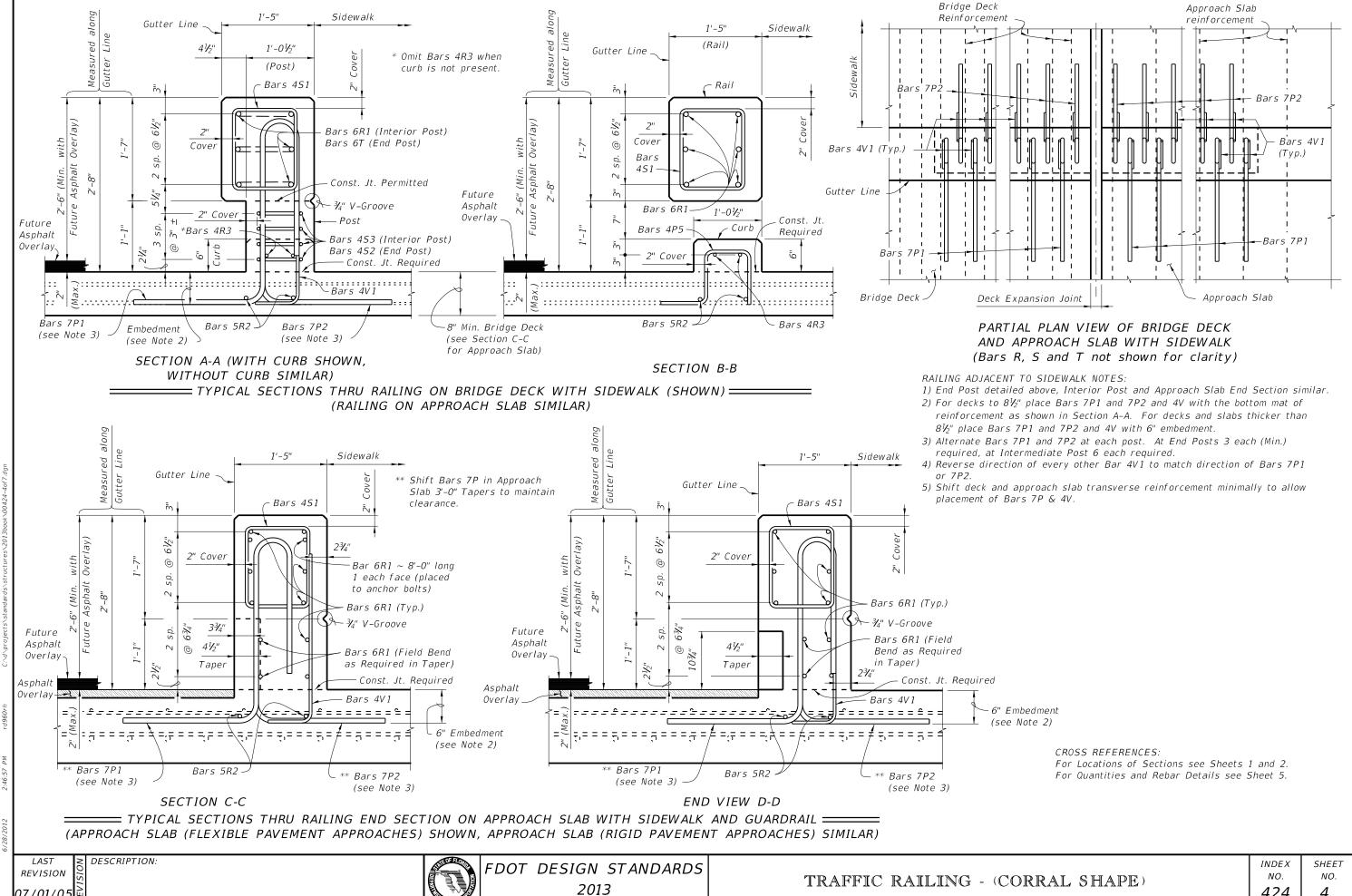
DESCRIPTION:



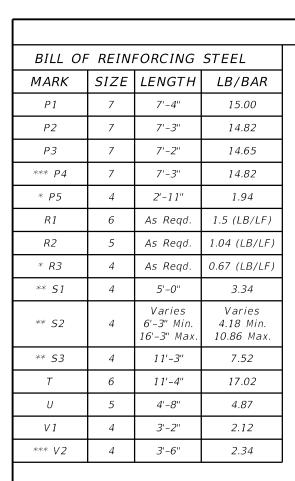




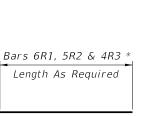




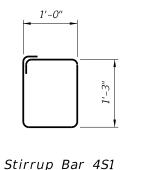
07/01/05

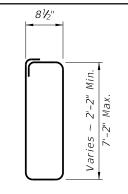


- * Bars 4P5 and 4R3 are to be used with a curb only.
- ** Bend Bars 4S1, 4S2 & 4S3 around a #3 Stirrup Pin.
- *** Bars 7P4 & 4V2 are to be used on C-I-P Concrete Retaining Walls.

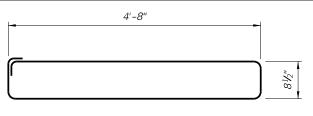


Bars 6R1, 5R2 & 4R3





CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

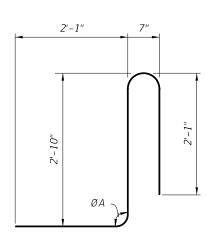


5'-4"

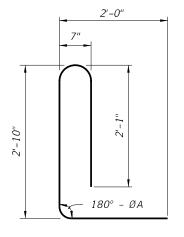
Stirrup Bar 4S3

Stirrup Bar 6T

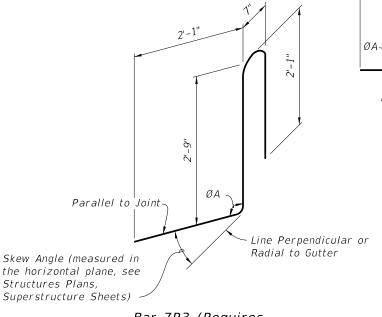
Stirrup Bar 4S2



Bar 7P1



Bar 7P2



8" 8½" ØA

2'-3" 2¾" ⊘ Pin

Bar 4V2 ***

Bar 4P5 * Stirrup Bar 5U

Top of C-I-P Concrete
Retaining Wall
adial to Gutter

Bar 4V1

Bar 7P3 (Requires 3 Dimensional Bend)

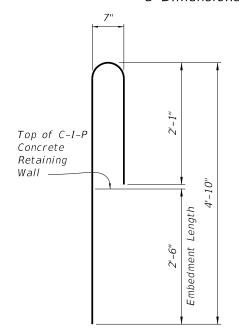
DESCRIPTION:

REINFORCING STEEL NOTES:
1. All bar dimensions in the bending diagrams are out to out.

- 2. The reinforcement for the railing on a C-I-P Concrete Retaining Wall shall be the same as detailed above for a 8" deck with ØA = 90°, where applicable. If bottom horizontal legs of Bars 7P1, 7P3 and 4V1 prohibit placement, Bars 7P4 and 4V2 may be substituted for Bars 7P1, 7P3 and 4V1 as shown.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.
- 4. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2'-6" Min. lap length for Bar 6R1, a 2'-0" Min. lap length for Bars 5R2 and a 1'-3" Min. lap length for Bars 4R3.
- 5. The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.

ROADWAY OR SIDEWALK	HIGH SIDE	LOW SIDE
CROSS-SLOPE	ØA	ØA
0% to 2%	90°	90°
2% to 6%	93°	87°
6% to 10%	96°	84°

ØA shall be 90° if Contractor elects to place Railing Perpendicular to the Deck.



ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	CONCRETE QUANTITY (CY)	REBAR QUANTITY (LB)
Typical 10'-0" Section w/Curb	1.13	451
Typical 10'-0" Section w/o Curb	1.03	428
Approach Slab with Guardrail End Section	0.14 (per LF)	44 (per LF)

Bar 7P4 ***

LAST REVISION 07/01/05



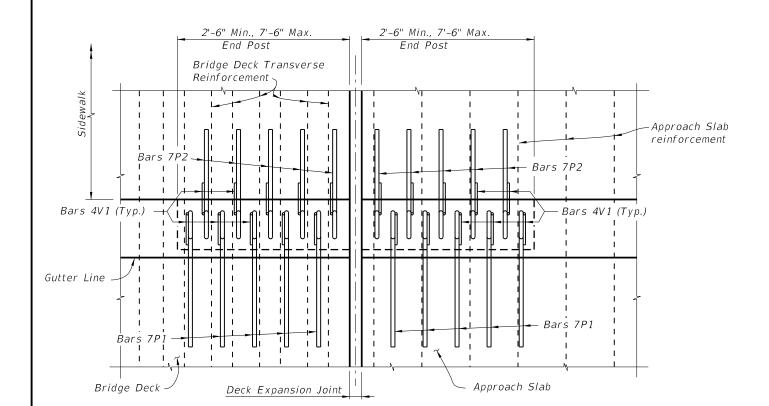
FDOT DESIGN STANDARDS 2013

TRAFFIC RAILING - (CORRAL SHAPE)

INDEX SHEET NO. NO. 424 5

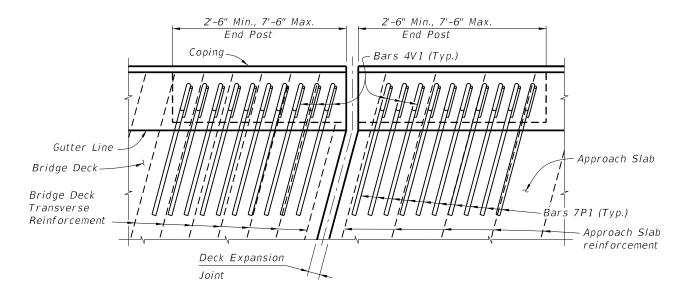
PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB - SKEW ANGLE GREATER THAN 15 DEGREES

- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter. BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):
- 3) End Post & Approach Slab End Section Place Bars 7P1 & 4V1 in obtuse corners of intersection of deck joint and gutter line. Place Bars 7P3 & 4V1 in acute corners of intersection of deck joint and gutter line as required. Interior Post - use Bars 7P1 and 4V1 placed with bottom mat of reinforcement. Shift deck or slab reinforcement minimally to allow proper placement of Bars 7P and 4V and to facilitate placement of concrete.
- APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 4) Place Bars 7P1 & 4V1 in obtuse corners of intersection of deck joint and gutter line and Bars 7P3 & 4V1 in acute corners of intersection of deck joint and gutter line as required. Shift deck or slab reinforcement minimally to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete.
- 5) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK - 0 DEGREE SKEW ANGLE

- 1) Alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 as detailed above to facilitate placement of concrete.
- 2) Shift deck transverse reinforcement minimally to allow placement of Bars 7P & 4V.



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB - SKEW ANGLE 15 DEGREES OR LESS

NOTES:

- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- 3) Bars 7P & 4V in the Approach Slab may be rotated to match Approach Slab reinforcement or placed perpendicular or radial to the gutter line.

BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):

- 4) Rotate vertical Bars 7P & 4V to match bridge deck reinforcement. Shift deck & slab transverse reinforcement to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete. APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 5) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.
- 6) Bars 7P at end of the railing shall be field cut and shifted to maintain clearance, see Railing End Taper Detail Sheet 2 for similar details.

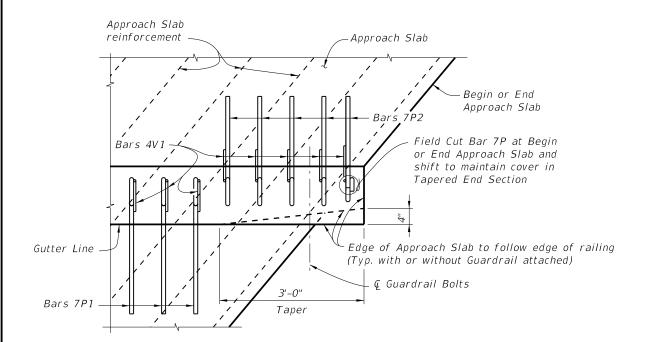
- 1) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at & Pier or Intermediate Bents are similar.
- 2) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.

LAST REVISION 07/01/05

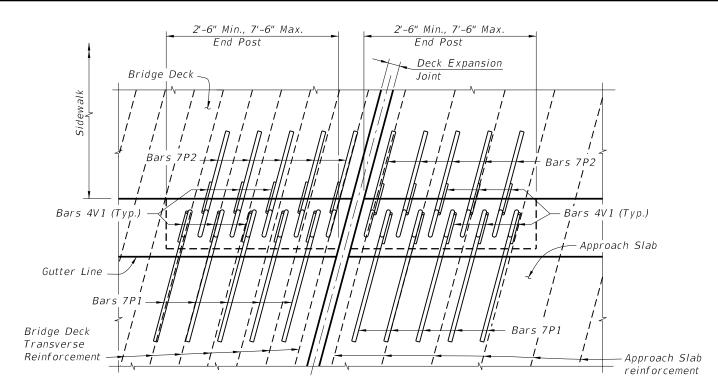
DESCRIPTION:



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK - SKEW ANGLE GREATER THAN 15 DEGREES



PARTIAL PLAN VIEW AT BEGIN OR END APPROACH SLAB WITH SIDEWALK AND RAILING WITH GUARDRAIL ATTACHED - SKEW ANGLE GREATER THAN 15 DEGREES SHOWN, 15 DEGREES OR LESS SIMILAR



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK - SKEW ANGLE 15 DEGREES OR LESS

- 1) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- 3) Edge of Approach Slab adjacent to the roadway shall follow end of railing, Bars 7P at end of the railing shall be field cut and shifted to maintain clearance, see detail bottom left this sheet for similar details. BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):
- 4) Alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 to facilitate placement of concrete.
- 5) Bars 7P & 4V shall be rotated to match bridge deck reinforcement. Shift deck transverse reinforcement minimally to allow placement of Bars 7P & 4V.
- 6) Railing End Post and reinforcement detailed above. Railing Interior Post reinforcement similar.
- APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 7) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.

NOTES:

- 1) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.

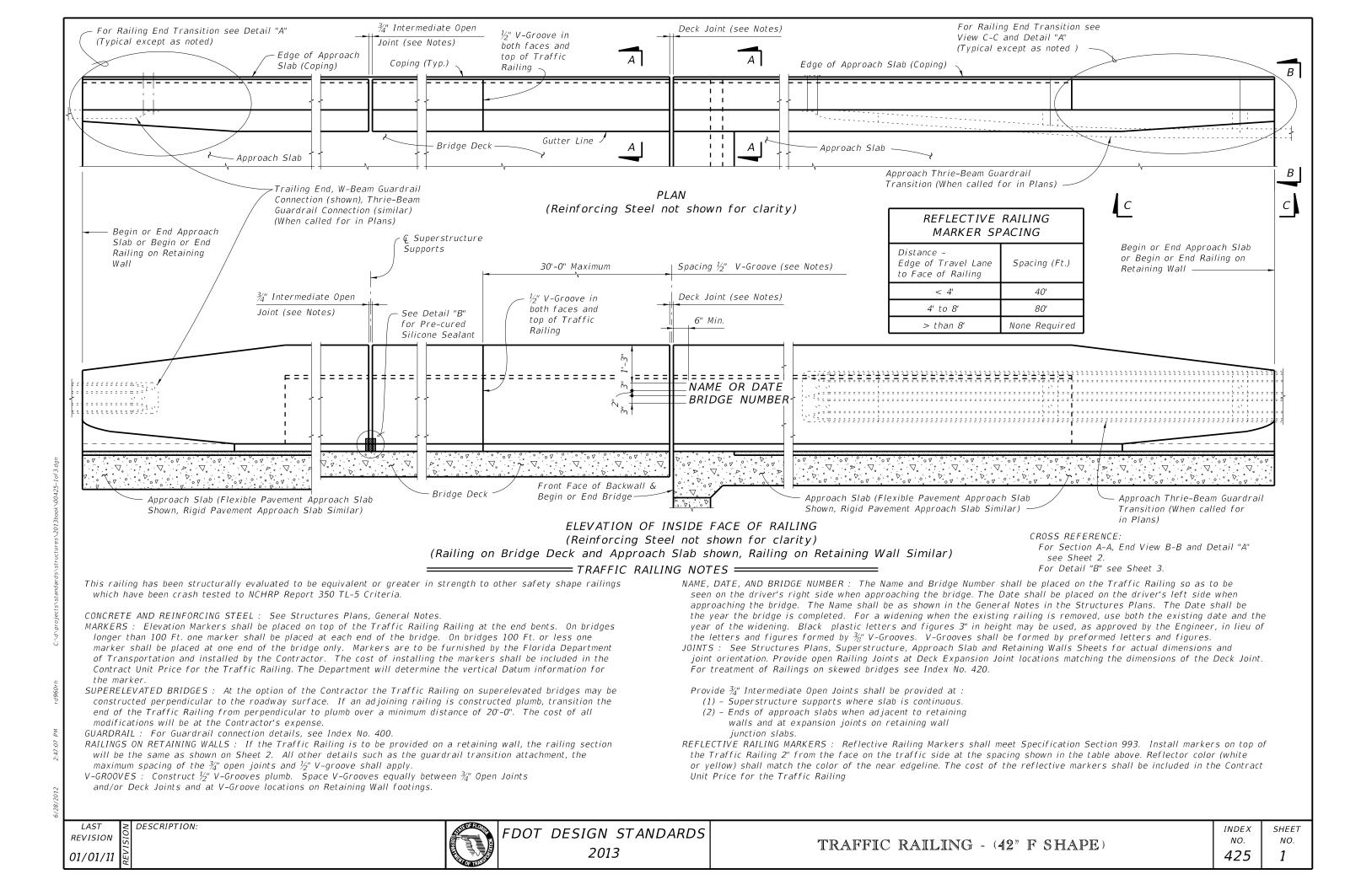
BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (NOT SHOWN):

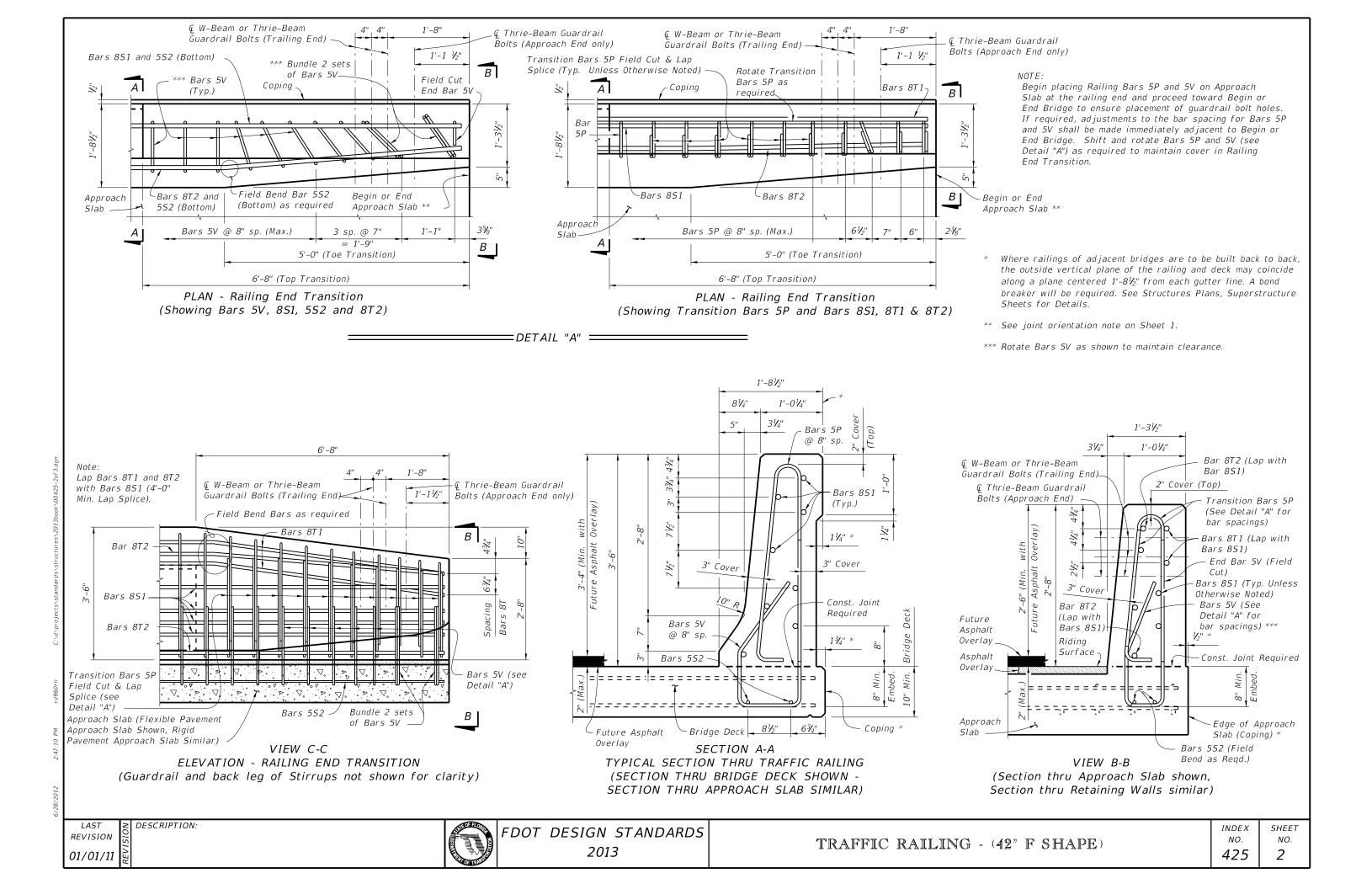
- 3) Deck transverse reinforcement may be shifted minimally as required to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete. Bars 7P1 & 4V1 or 7P2 & 4V1 shall be used on opposing sides of the joint depending on the direction of the skew, see Detail above. Approach Slab reinforcement may be shifted if conflicts occur.
- 4) Interior Post alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 to facilitate placement of concrete.
- 5) End Post alternate Bars 7P1 with Bars 7P2 and reverse direction of Bars 4V1 (as detailed) where possible. APPROACH SLAB WITH GUARDRAIL ATTACHED (SHOWN):
- 6) Use Bars 7P2 and reverse direction of Bars 4V1 where skew restricts use of Bars 7P1 & 4P1.
- 7) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.

LAST REVISION 07/01/05

DESCRIPTION:

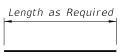


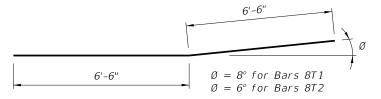




ROADWAY	LOW GUTTER		HIGH GUTTER	
CROSS-SLOPE	ØA	ØB	ØA	ØB
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	93°
6% to 10%	96°	84°	84°	96°

ØA and ØB shall be 90° if Contractor elects to place Railing perpendicular to the Deck.





BARS 851 & 552

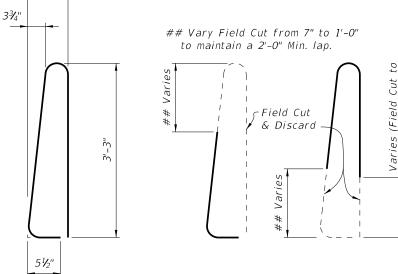
1. All bar dimensions in the bending diagrams are out to out.

as detailed above for a 10" deck with $\emptyset A = \emptyset B = 90^{\circ}$.

2. The reinforcement for the railing on a retaining wall shall be the same

3. All reinforcing steel at the open joints shall have a 2" minimum cover. 4. Bars 851 may be continuous or spliced at the construction joints. Lap splices for Bars 851 and 552 shall be a minimum of 4'-0" and 2'-0", respectively. 5. The Contractor may utilize Welded Wire Reinforcement when approved by the

Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



TRANSITION STIRRUP BARS 5P To Be Field Cut (10 of each required per Railing End Transition)

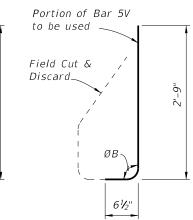
Contractor's option 45° 54°30′ ØB. 1'-01/2"

STIRRUP BAR 5V

END STIRRUP BAR 5V

Railing End Transition)

TRANSITION BARS 8T1 & 8T2 (2 of each required per Railing End Transition)



To Be Field Cut (One required per

LAST REVISION 01/01/11

STIRRUP BAR 5P

REINFORCING STEEL NOTES:

DESCRIPTION:





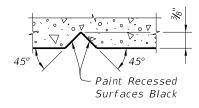
INDEX SHEET NO. NO.

DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

INTERMEDIATE JOINT SEAL NOTES:

Pre-cured Silicone Sealant (4" wide)

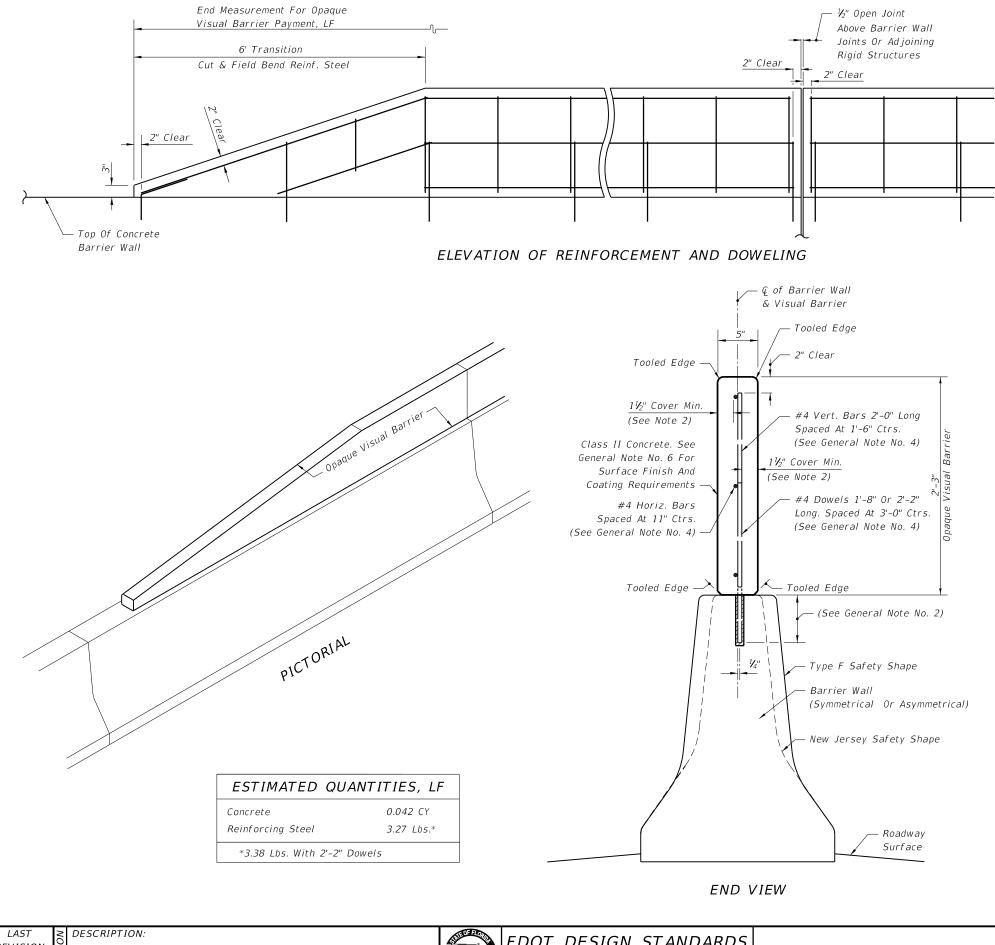
- 1. At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant in accordance with Specification Section 932.
- 2. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.
- 3. The cost of the Pre-cured Silicone Sealant shall be included in the Contract Unit Price for the Traffic Railing.



SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.154	
Reinforcing Steel	LB/LF	44.71	

The estimated railing quantities are based on a 2% deck cross slope; railing on low side of deck.



GENERAL NOTES

- 1. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when yielding to such strikes.
- 2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved chemical grout. Embedment holes shall be \%" diameter, drilled to a depth \%" below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the 1½" Cover Minimum to the face of the Opaque Visual Barrier.

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in the plans.

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

- 4. In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer.

 Details shall be submitted with requests for substitution.
- 5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineer's approval.

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations in Note No. 1 above.

- 6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finish Coating in accordance with Section 400 only when called for in the plans.
- 7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.

LAST REVISION 07/01/07



This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be 3/4" by $2\frac{1}{2}$ " slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM 449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for $\frac{1}{8}$ 0 anchor bolts; 55,000 lbs. for the $\frac{1}{4}$ anchor bolts with 13" embedment; and 30,500 lbs. for the $\frac{1}{4}$ 0 anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1\frac{1}{2}" spaces.

Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints.

Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post.

The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

NEOPRENE PADS: Neoprene pads must be plain pads with a durometer hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required.

ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The Department will determine the vertical Datum information for the marker.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall conform to Section 993 of the Specifications. Install markers at the top of the guardrail posts at the spacings shown in the table below. Reflector color (white or yellow) shall conform to the color of the near edgeline.

PEDESTRIAN SAFETY TREATMENTS: Pedestrian Safety Treatment is required when called for in the Plans. See Index No. 400 for details.

BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. Transition Blocks and Curbs, Bridge Name Plate, Reflective Railing Markers and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

REFLECTIVE RAILING MARKER SPACING		
Distance – Edge of Travel Lane to Face of Railing	Spacing (Ft.)	
< 4'	40'	
4' to 8'	80'	
> than 8'	None Required	

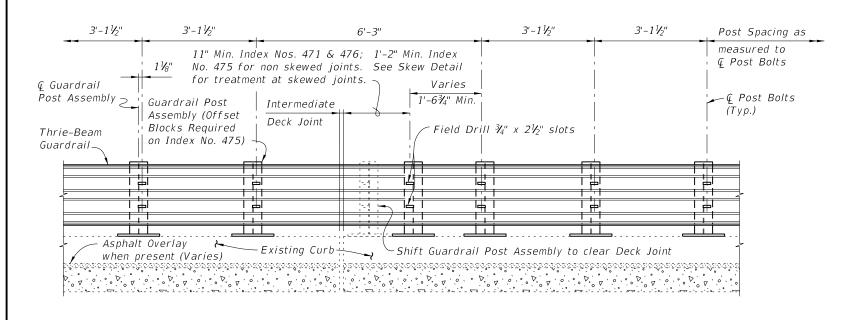
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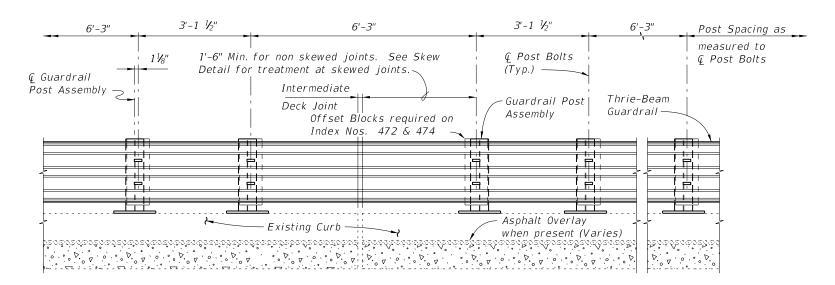
REVISION

01/01/08

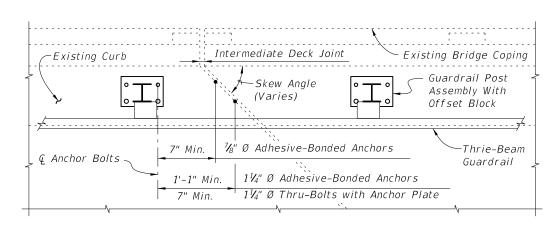
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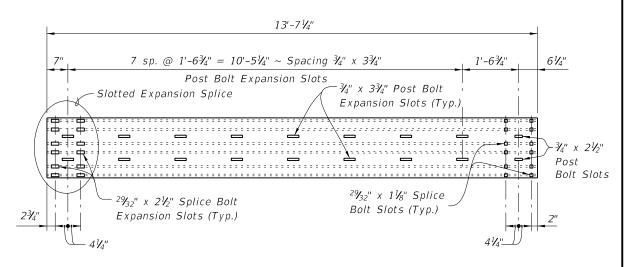
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX NOS. 471, 475 & 476



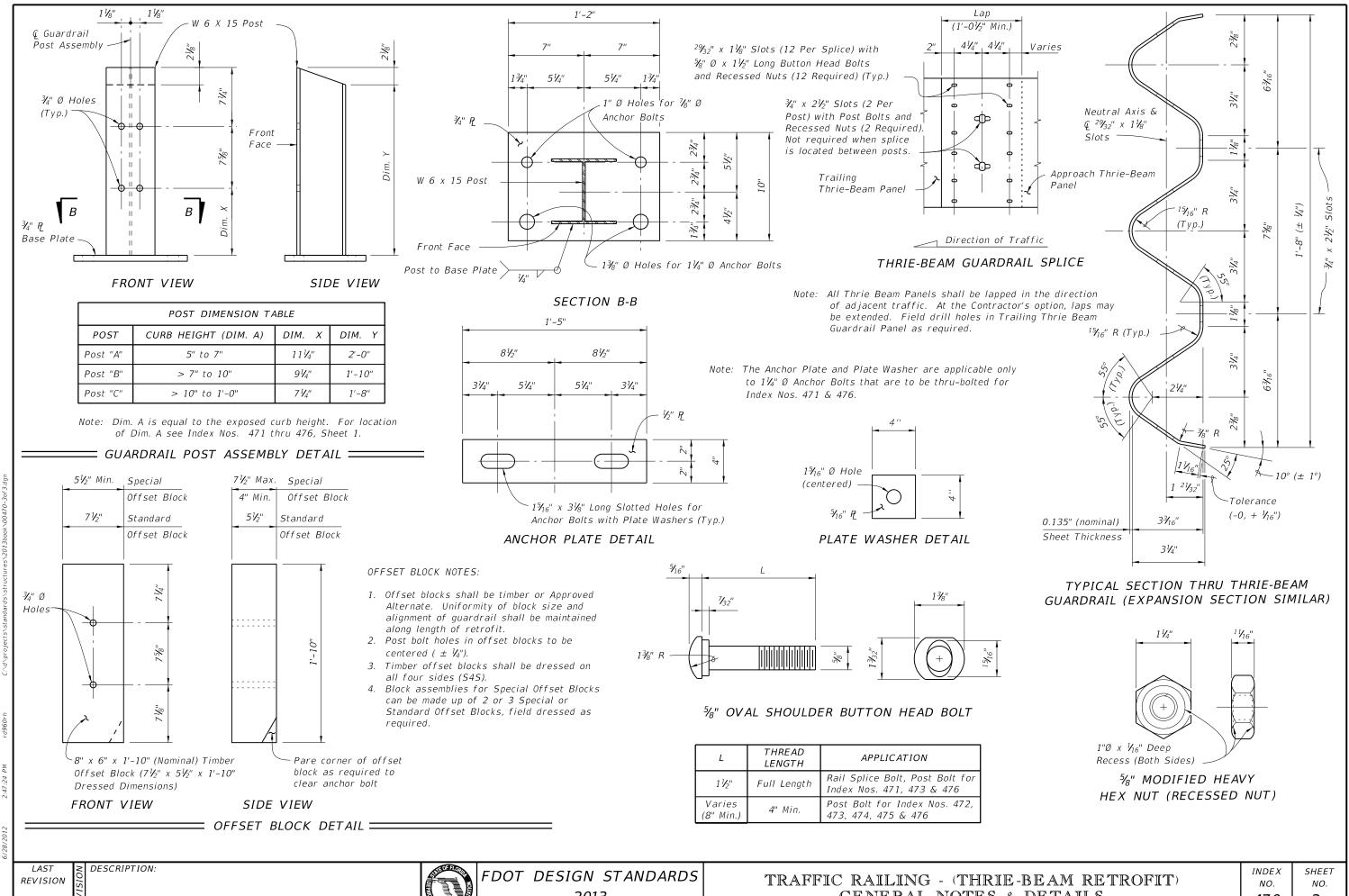
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX NOS. 472, 473 & 474



PARTIAL PLAN INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION



01/01/08

For Traffic Railing Notes and Details

see Index No. 470.

NOTES:

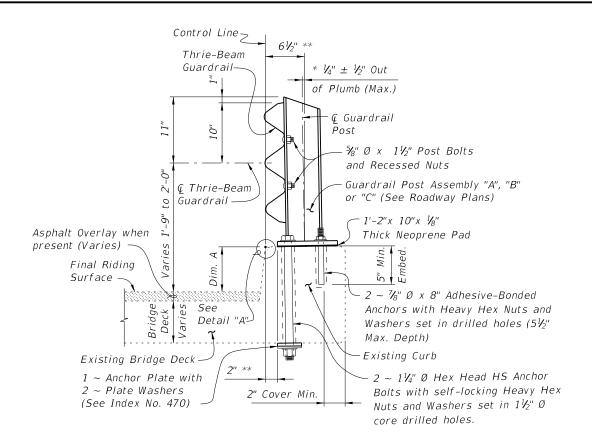
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2,
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be removed off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2.

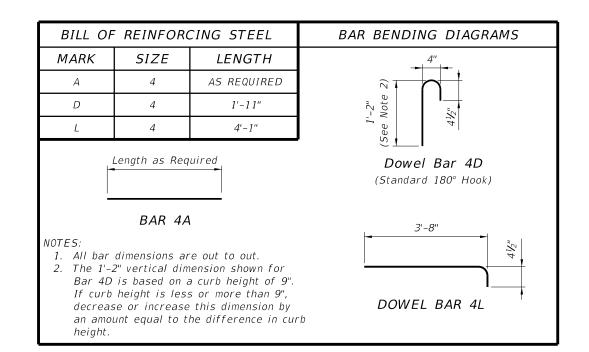
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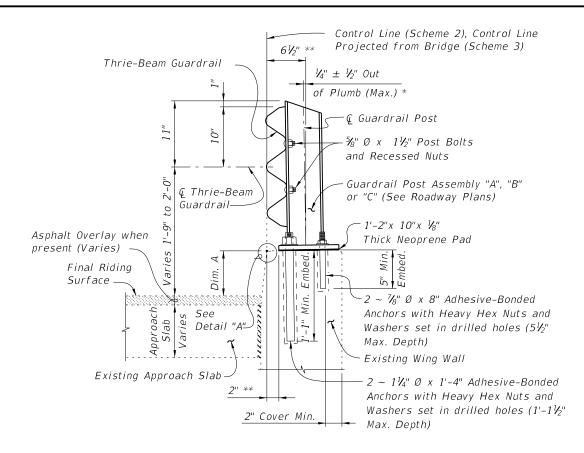
FDOT DESIGN STANDARDS 2013

REVISION 01/01/08



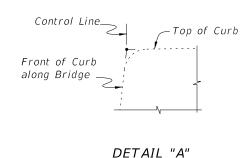
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



Curb Existing (Wir to

TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

CROSS REFERENCES:

For location of Section A-A see Sheets 1. 3 & 4. For location of Section B-B see Sheets 3 & 4. For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.

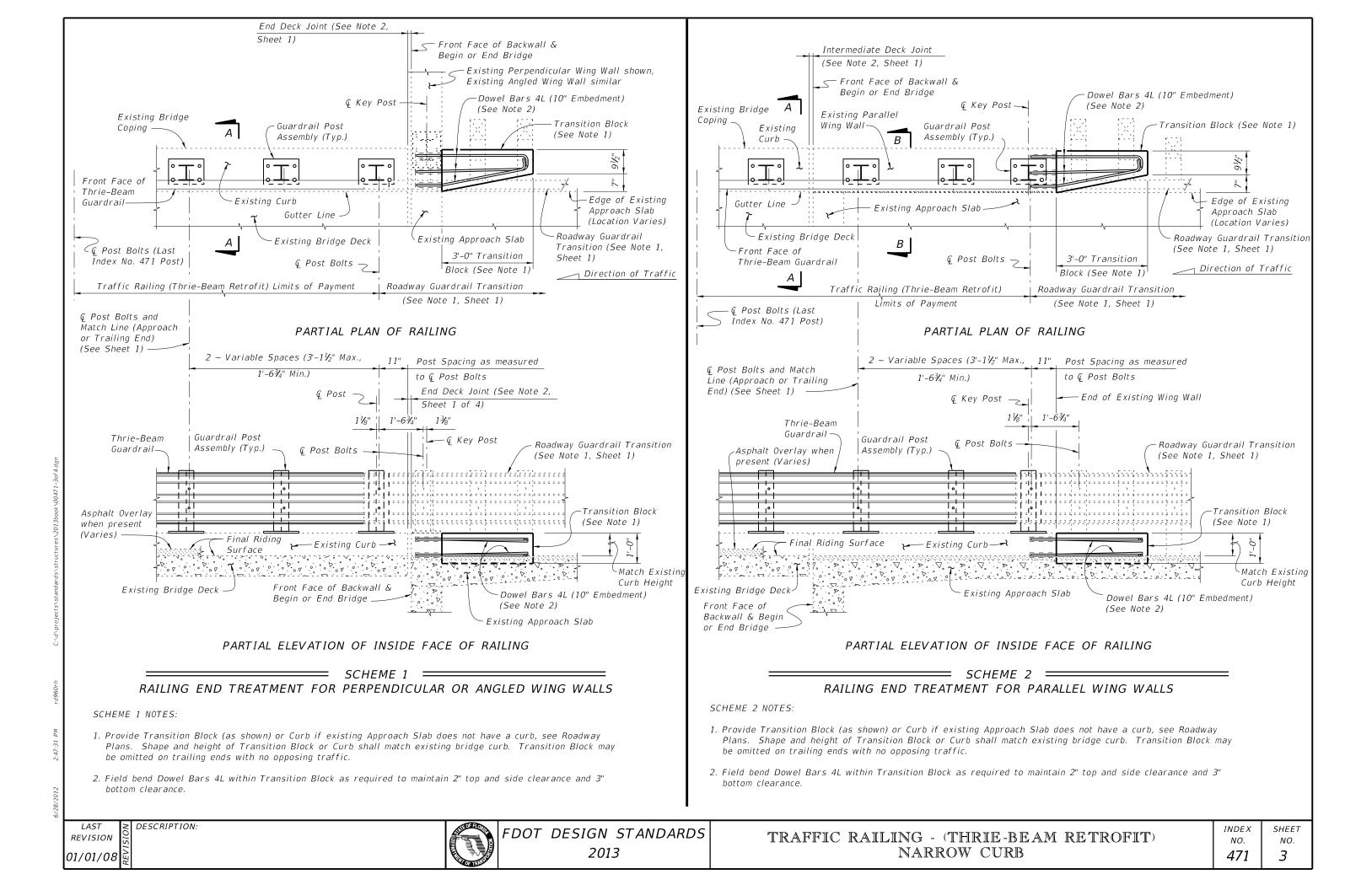
FDOT DESIGN STANDARDS

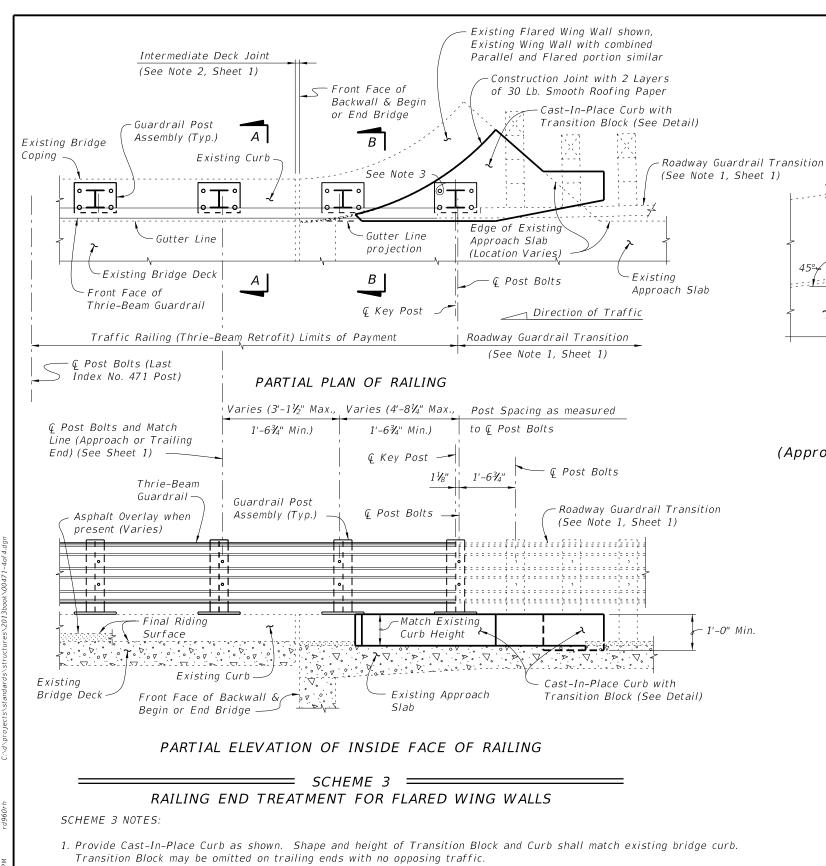
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) NARROW CURB

INDEXNO. 471

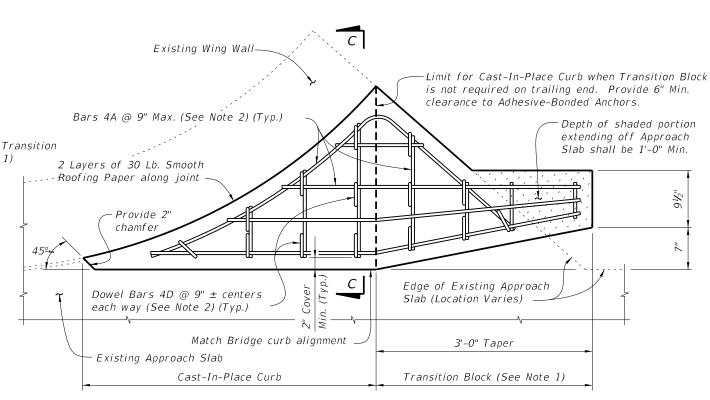
SHEET NO.

DESCRIPTION: LAST REVISION

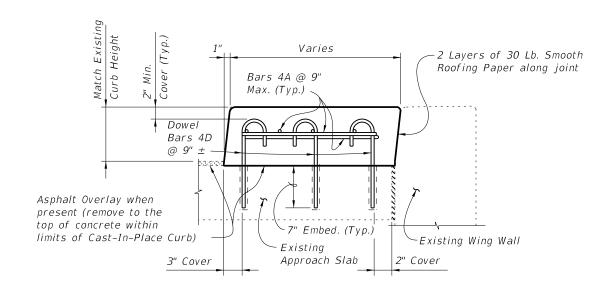




- 2. Field cut and bend Bars 4A and rotate Dowel Bars 4B within Curb and Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. A single $\frac{V_8}{8}$ Ø x 8" Adhesive-Bonded Anchor may be omitted as shown when 2" clear cover cannot be provided.



PLAN OF CAST-IN-PLACE CURB & TRANSITION BLOCK DETAIL (Approach End with Transition Block Shown, Trailing End without Transition Block Similar)



SECTION C-C



NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2.

For Traffic Railing Notes and Details

see Index No. 470.

DESCRIPTION: REVISION 01/01/08

FDOT DESIGN STANDARDS 2013

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 1

INDEX NO. 472

SHEET NO.

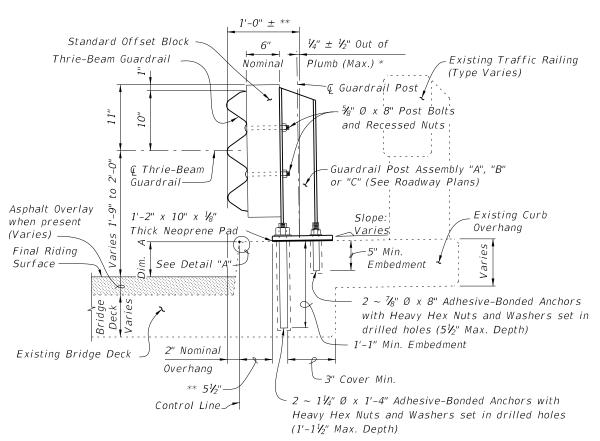
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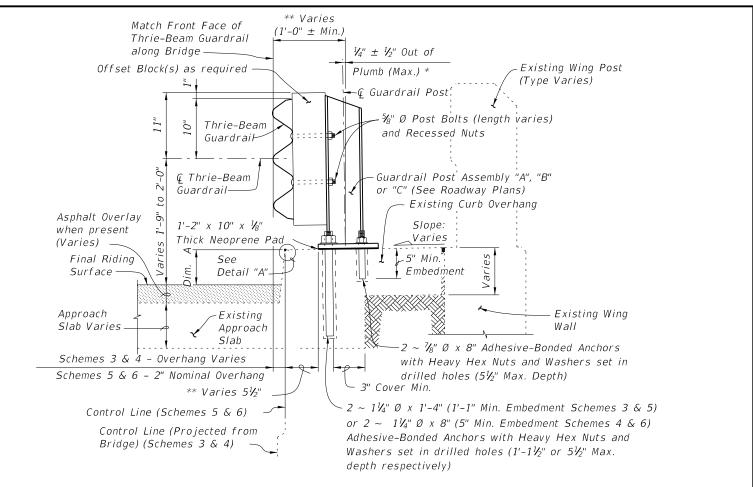
REVISION

07/01/08

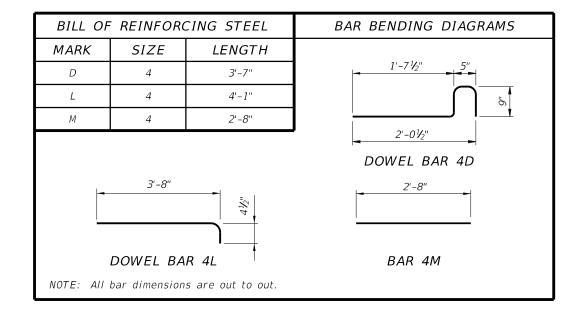
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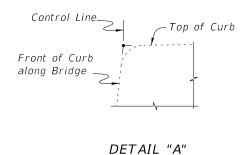
SECTION A-A
TYPICAL SECTION THRU RAILING ON BRIDGE DECK

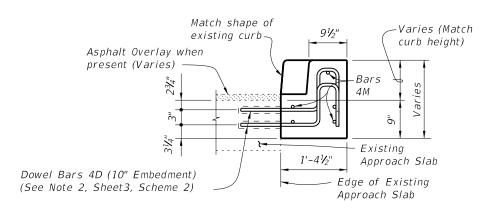


SECTION B-B
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB
(SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)



- Shim with washers around Anchors as required to maintain tolerance.
- Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

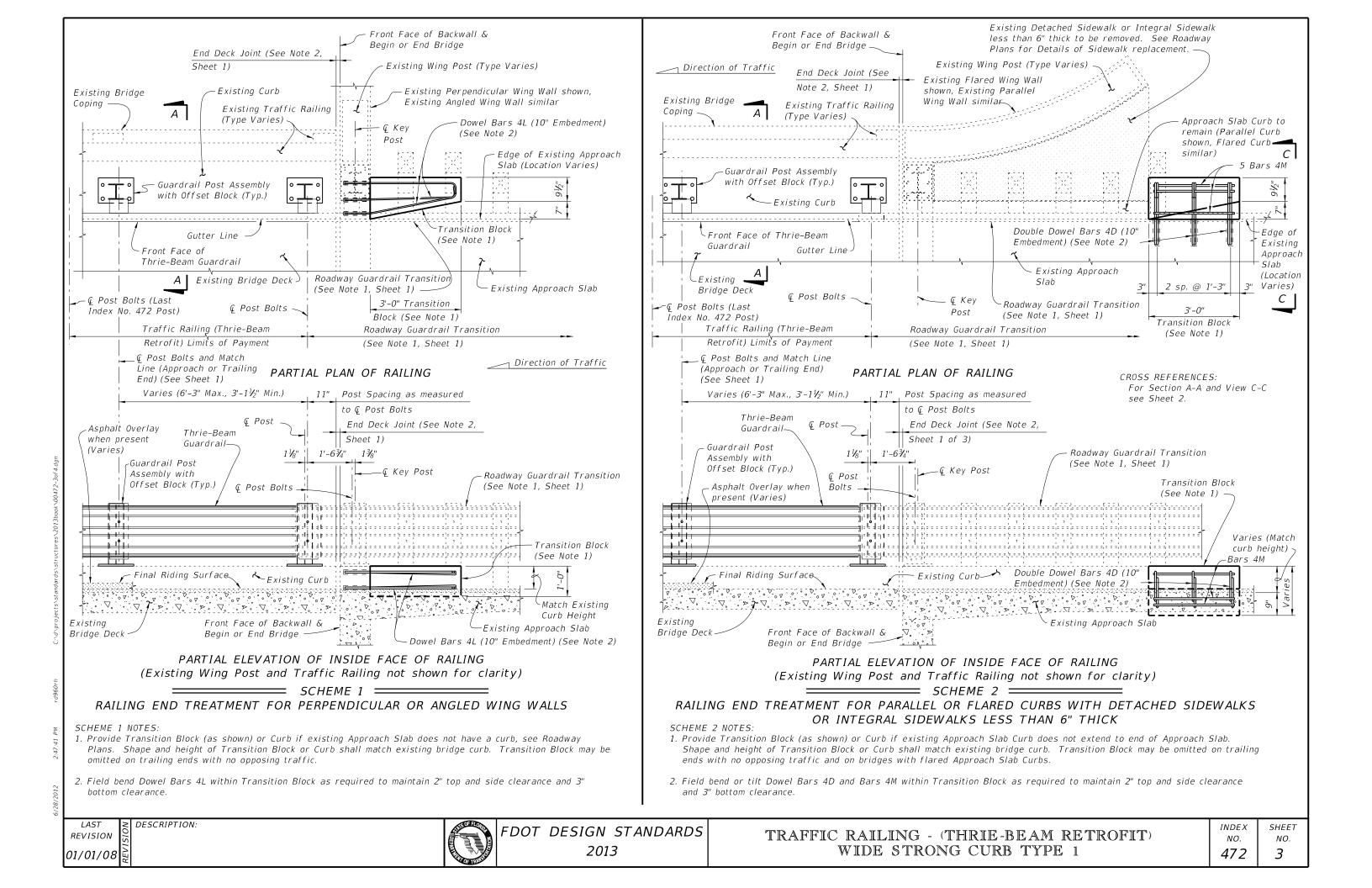
For location of Section B-B see Sheet 4.

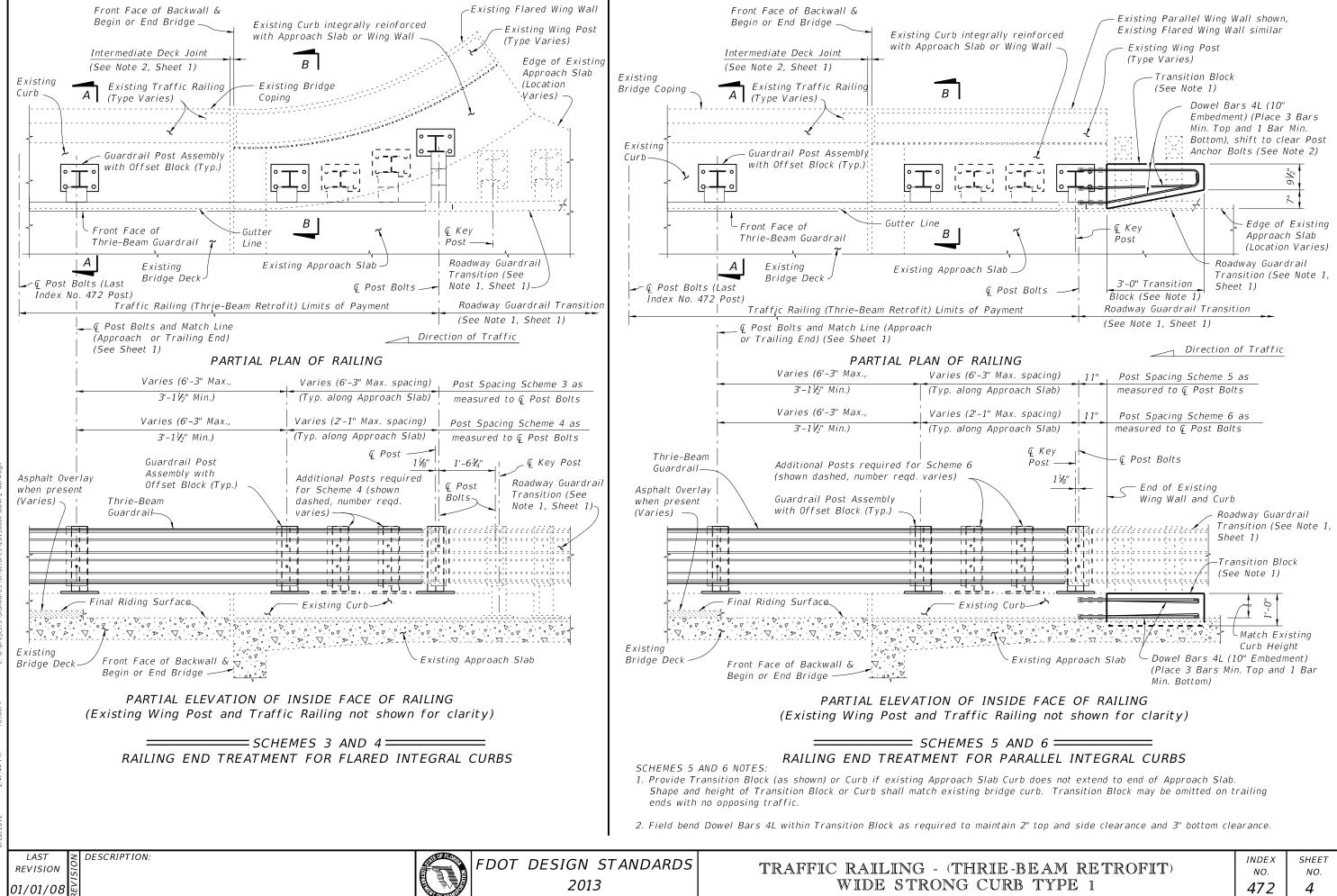
For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.

FDOT DESIGN STANDARDS
2013

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)
WIDE STRONG CURB TYPE 1





(Existing Traffic Railing not shown for clarity)

TYPICAL TREATMENT OF RAILING ALONG BRIDGE

NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

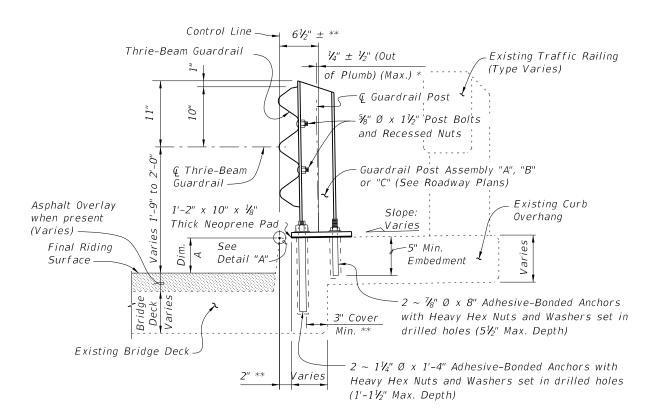
LAST REVISION 01/01/08



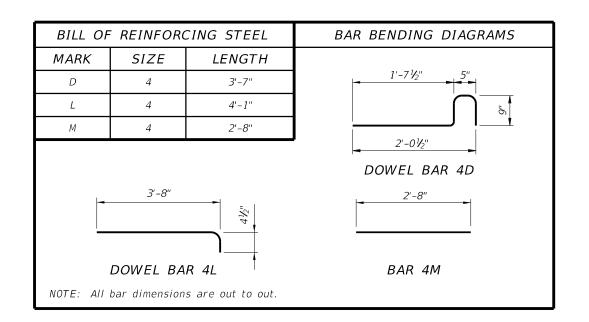
FDOT DESIGN STANDARDS 2013

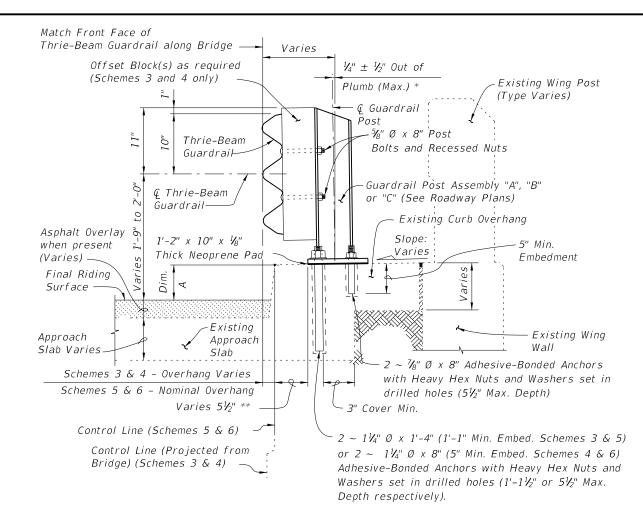
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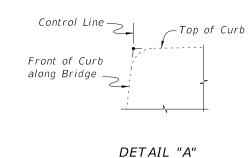
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

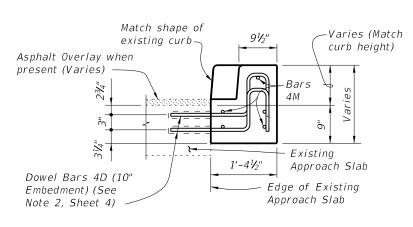




SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- * Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- ** Offset may vary ± 1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index No. 470.

For application of Dim. A see Post Dimension Table

on Index 470. Sheet 3.

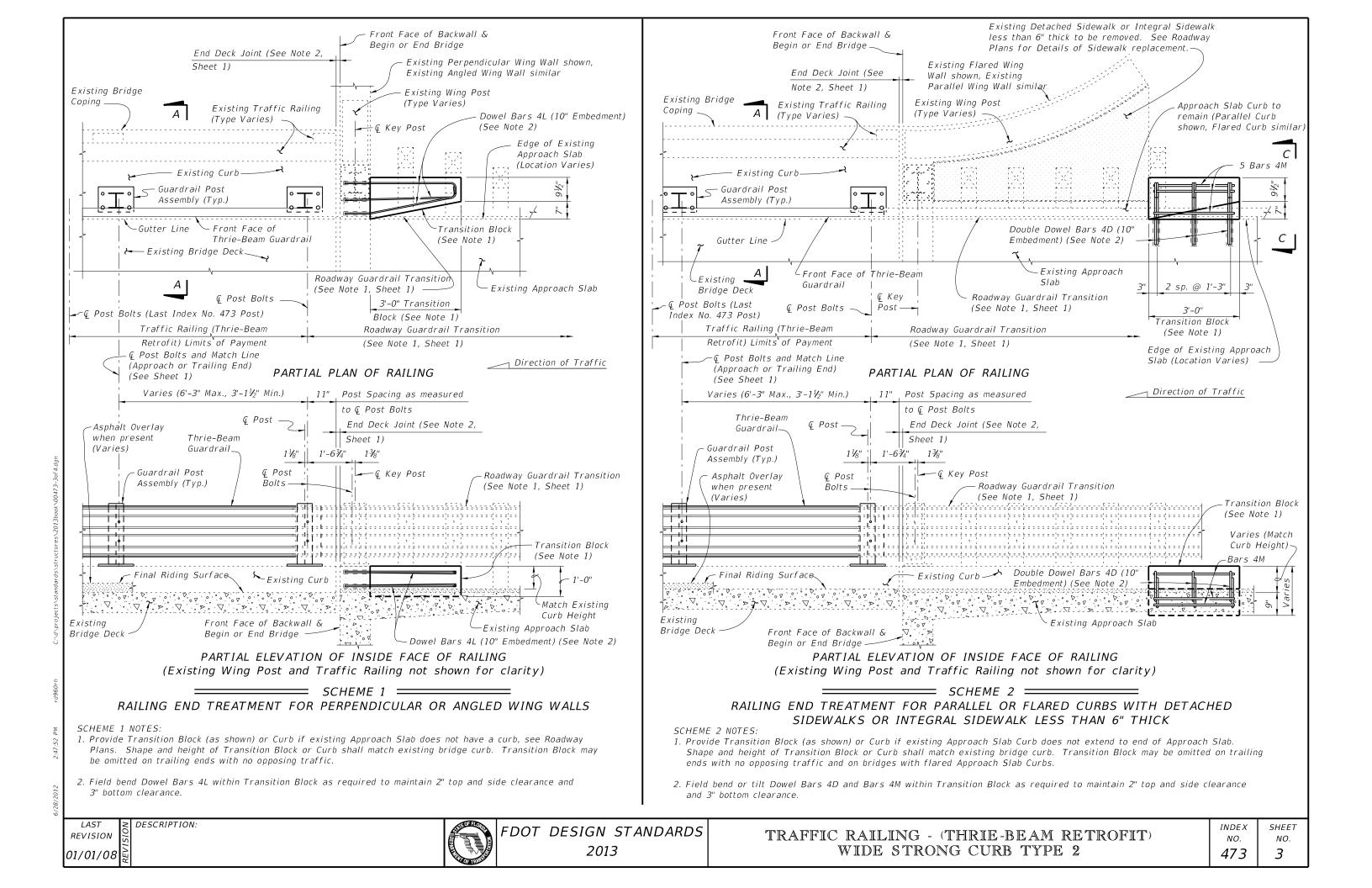
FDOT DESIGN STANDARDS

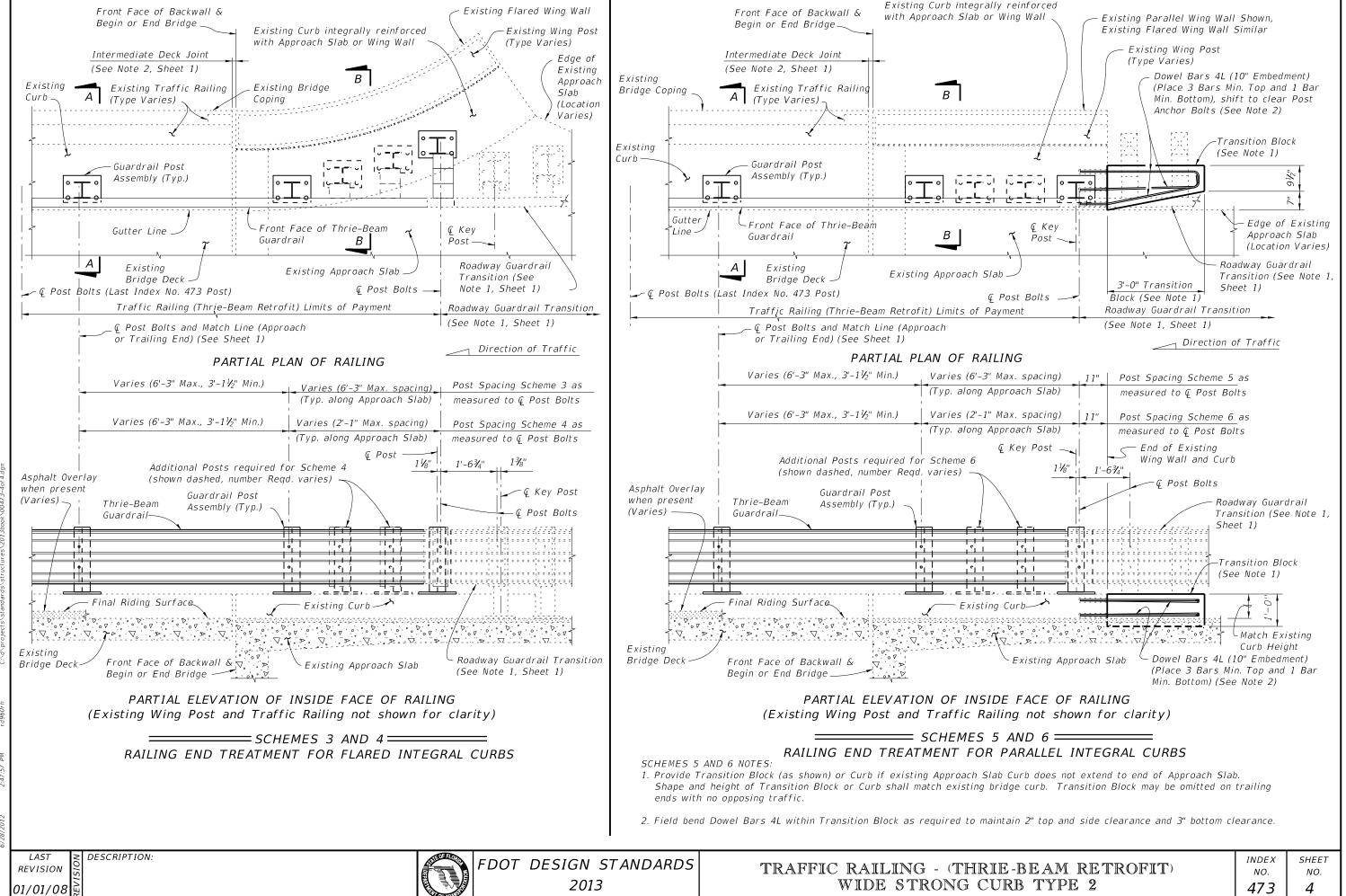
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE STRONG CURB TYPE 2

INDEX NO. 473

SHEET NO. 2

DESCRIPTION: LAST REVISION





WIDE STRONG CURB TYPE 2

4

==== TYPICAL TREATMENT OF RAILING ALONG BRIDGE ====

NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES: For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

DESCRIPTION: 01/01/08

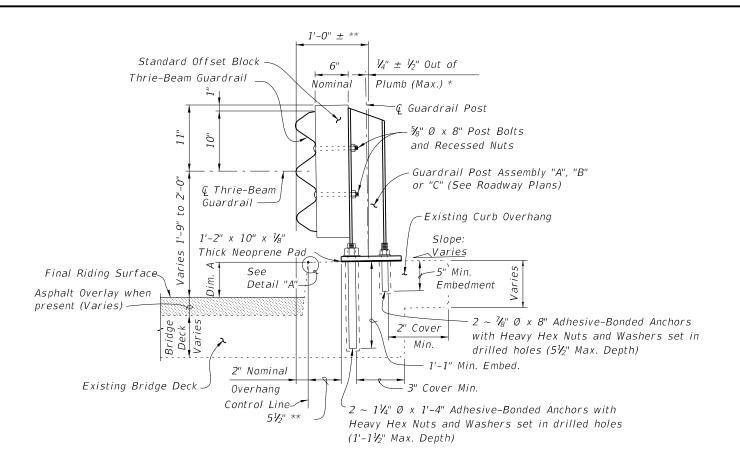
FDOT DESIGN STANDARDS 2013

LAST

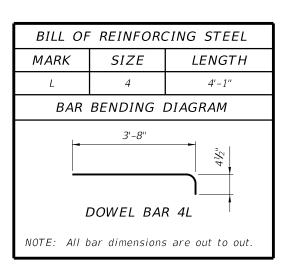
REVISION

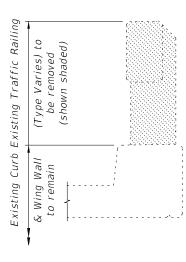
INDEX NO.

DESCRIPTION:

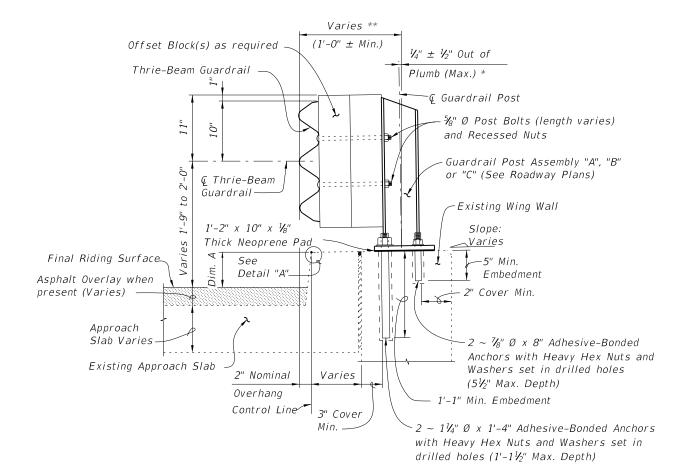


SECTION A-A
TYPICAL SECTION THRU RAILING ON BRIDGE DECK



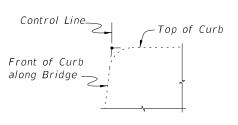


TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION B-B (SCHEME 2)
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

*Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

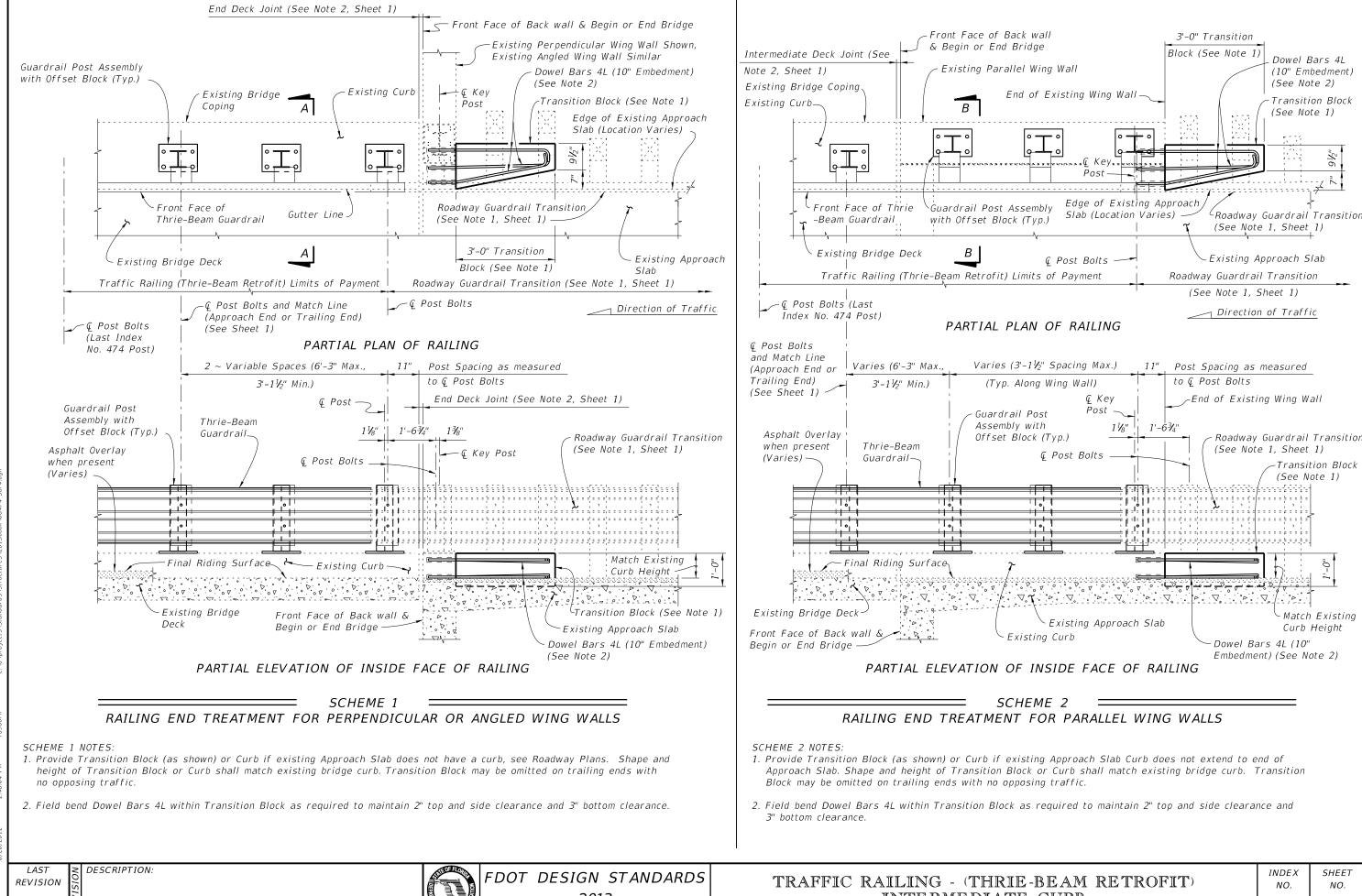


DETAIL "A"

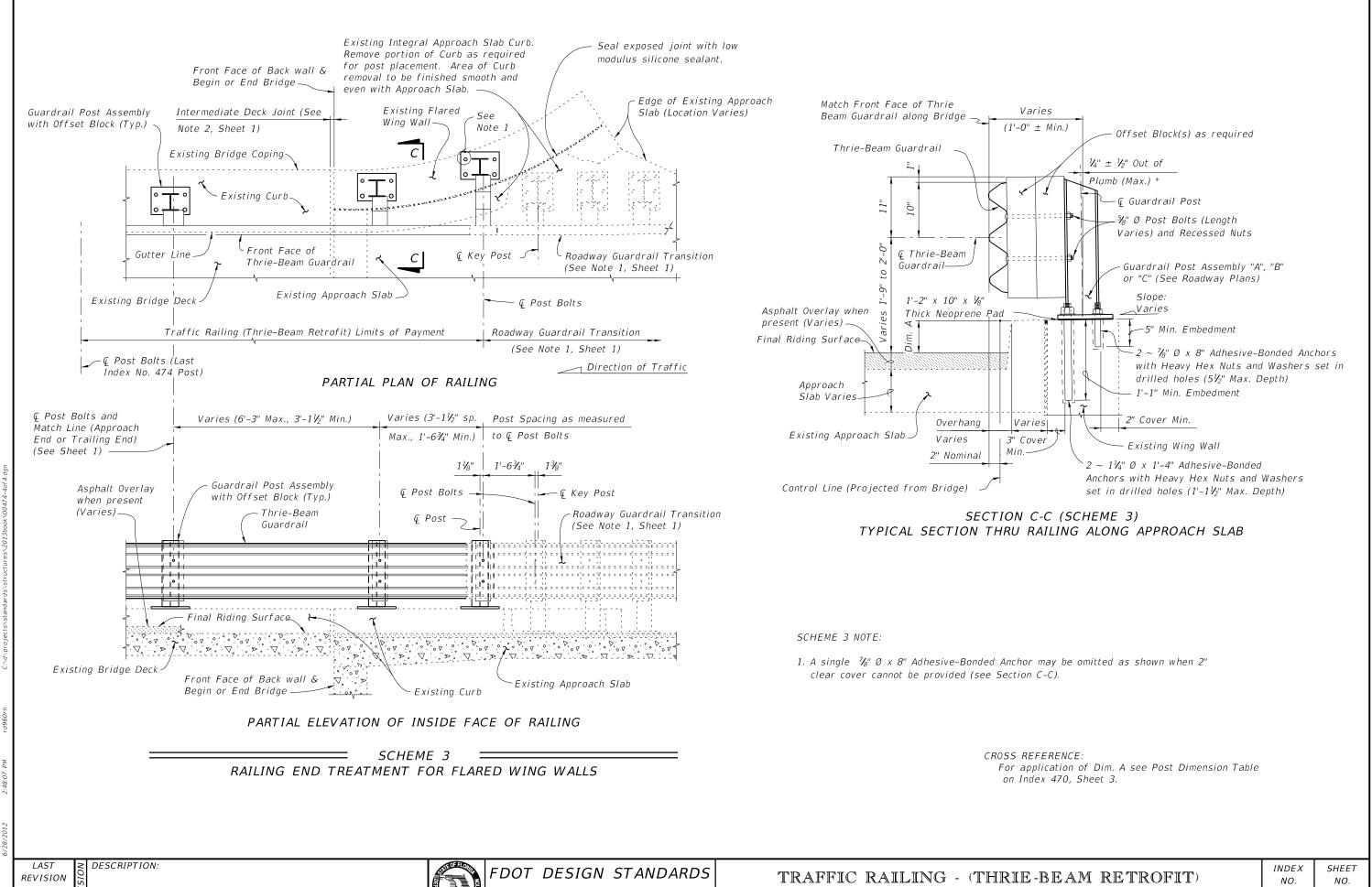
CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3.
For location of Section B-B see Sheet 3
For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.

^{***}Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.







(Existing Traffic Railing not shown for clarity)

=== TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

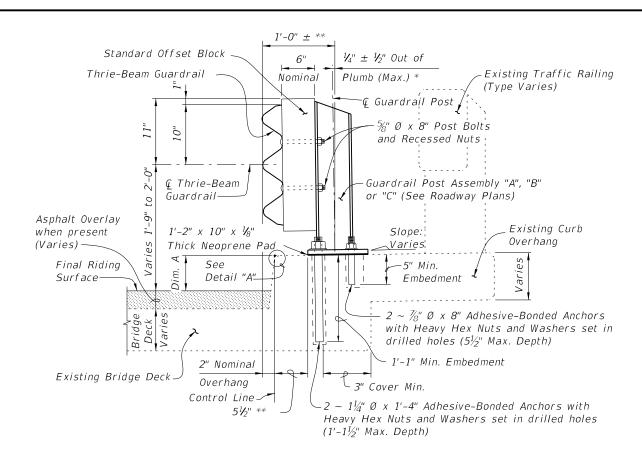
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

DESCRIPTION: LAST REVISION 01/01/08

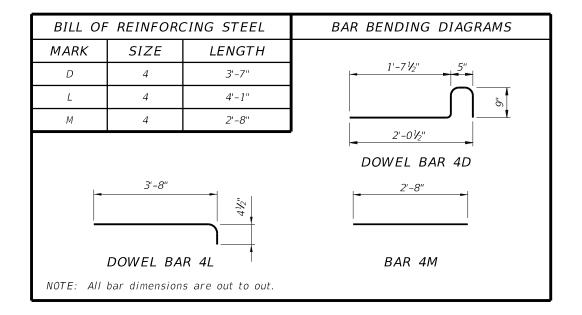


FDOT DESIGN STANDARDS 2013





SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

Final Riding

Surface

Approach

Slab Varies

Schemes 3 & 4 - Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

Control Line (Schemes 5 & 6) -

Control Line (Projected from

Bridge) (Schemes 3 & 4) -

when present

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required

Thrie-Beam

Guardrail-

ℂ Thrie-Beam

1'-2" x 10" x 1/8"

See

Thick Neoprene Pad

Detail "A".

Existing

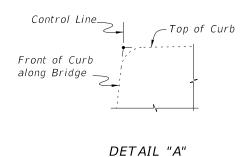
Approach

Slab

Varies 51/2" **

Guardrail—

 $^{\circ}$ Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



Match shape of -Varies (Match 91/2" existing curbcurb height) Asphalt Overlay when present (Varies) Bars 4M Existing 1'-41/2" Approach Slab Dowel Bars 4D (10" Embedment) Edge of Existing (See Note 2, Sheet 3) Approach Slab

VIEW C-C

CROSS REFERENCES:

Varies ** $(1'-0'' \pm Min.)$

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

Plumb (Max.) *

-Ç Guardrail Postî

%" Ø Post Bolts (length varies)

-Guardrail Post Assembly "A", "B"

Existing Curb Overhang

or "C" (See Roadway Plans)

and Recessed Nuts

Slope:

Varies

Embedment

_5" Min.

∽ 3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

Existing Wing Post

Existing Wing

Wall

~ \%" \@ x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5)

or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}")$ or $5\frac{1}{2}"$ Max.

drilled holes $(5\frac{1}{2}"$ Max. Depth)

(Type Varies)

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.

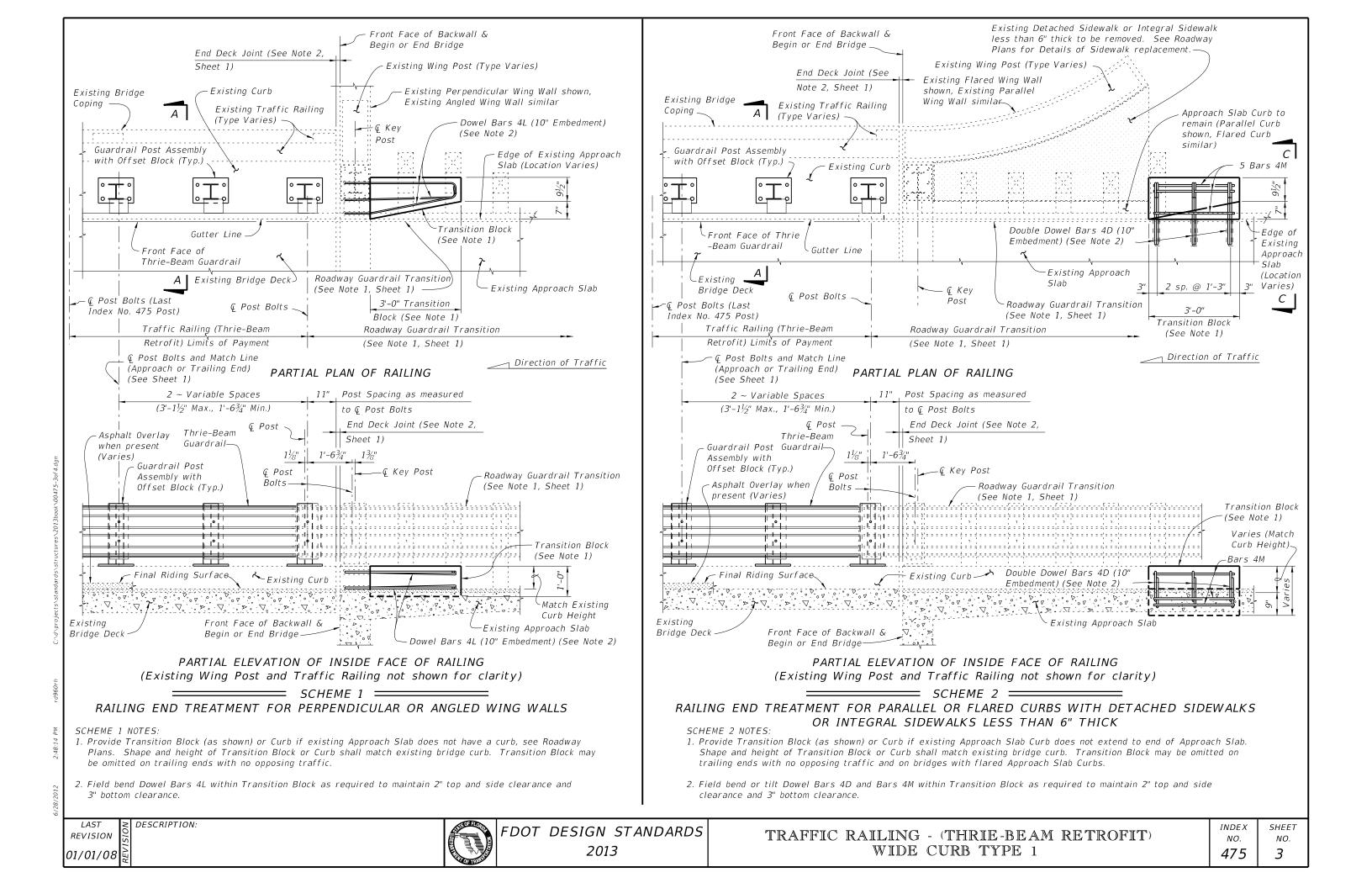
FDOT DESIGN STANDARDS 2013

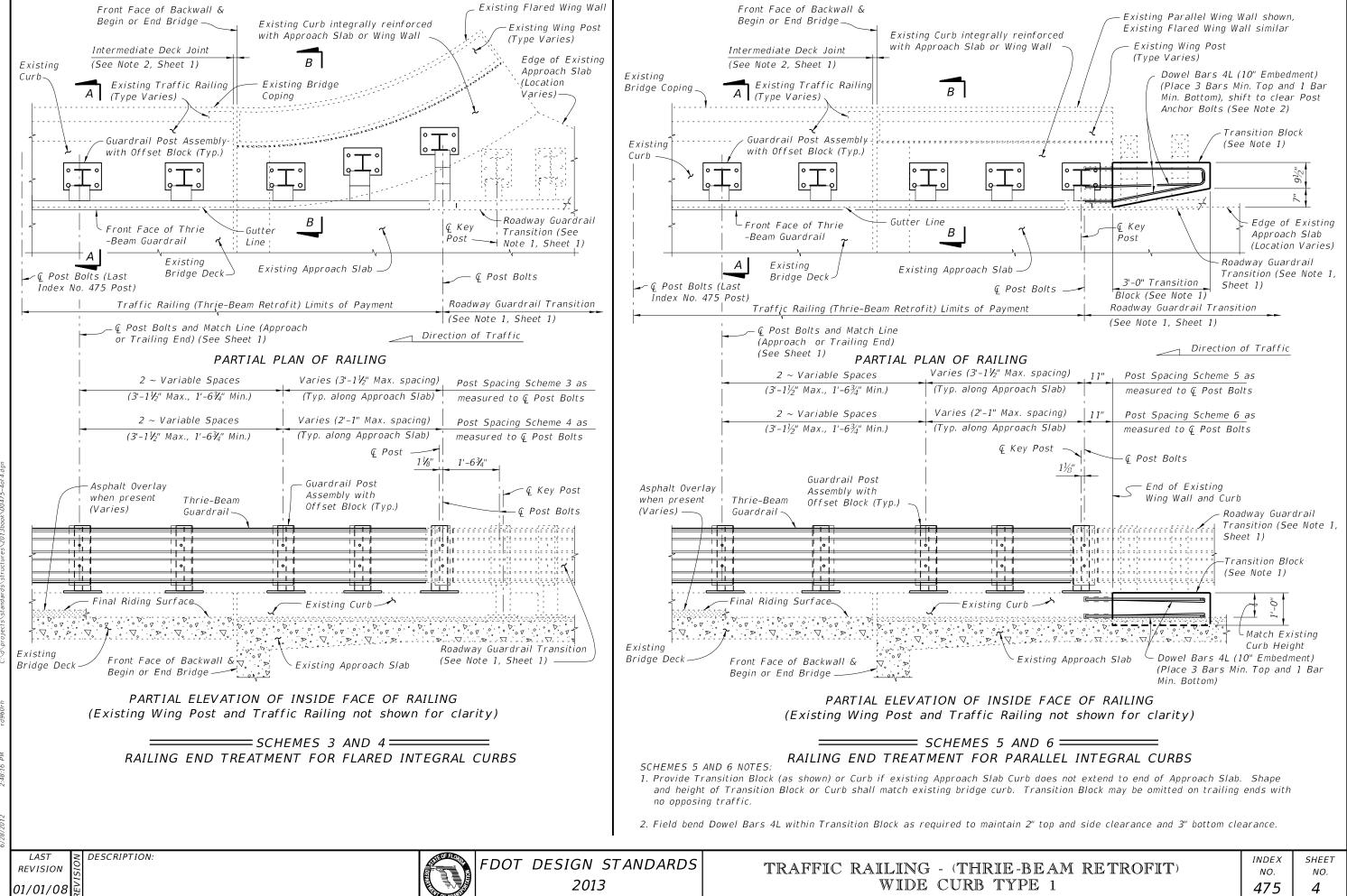
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 1

INDEX NO. 475

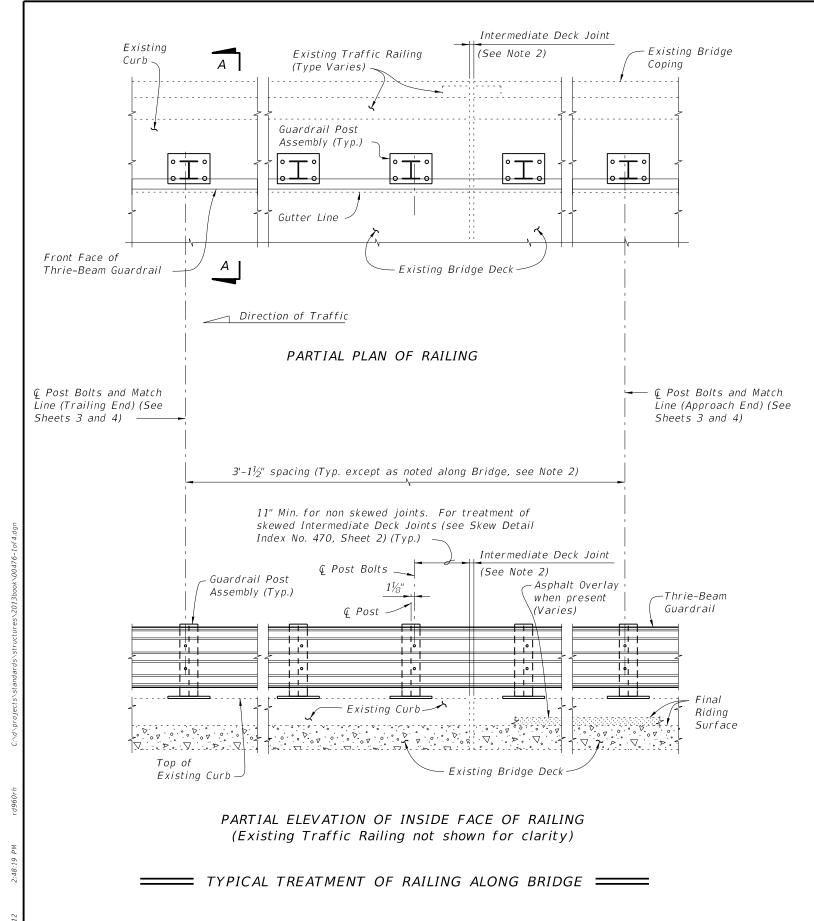
SHEET NO. 2

DESCRIPTION: LAST REVISION





WIDE CURB TYPE 1



NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.

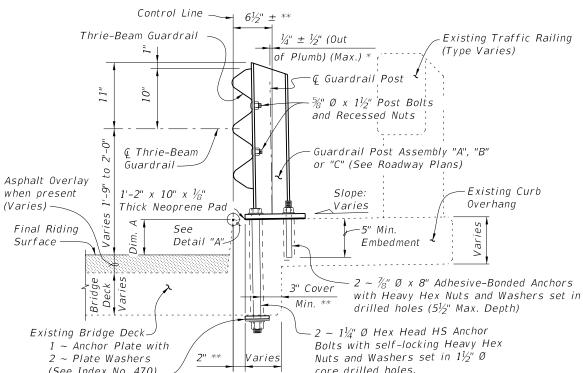
LAST REVISION 01/01/08

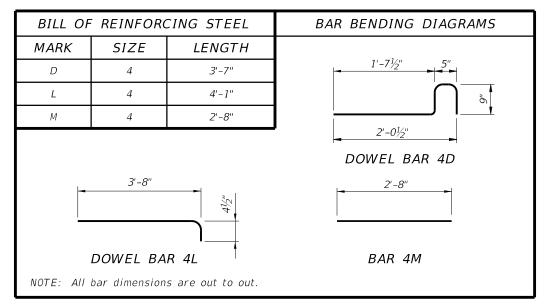
DESCRIPTION:

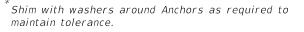


FDOT DESIGN STANDARDS
2013

DESCRIPTION:







Match Front Face of

Asphalt Overlay

Final Riding

Surface

Approach

Slab Varies-

when present

(Varies) -

Thrie-Beam Guardrail along Bridge 🥌

(Schemes 3 and 4 only)

Offset Block(s) as required

Thrie-Beam

Guardrail-

⊊ Thrie-Beam

1'-2" x 10" x 1/3"

Thick Neoprene Pad

-Existing

Approach

Slab

Varies 51/5" **

Guardrail -

Schemes 3 & 4 - Overhang Varies

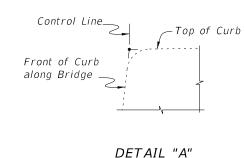
Schemes 5 & 6 - Nominal Overhang

Control Line (Schemes 5 & 6) _

Control Line (Projected from

Bridge) (Schemes 3 & 4) —

 $[\]hat{O}$ Offset may vary \pm 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



Match shape of -Varies (Match existing curbcurb height) Asphalt Overlay when present (Varies) 4MExisting Approach Slab Dowel Bars 4D (10" Embedment) Edge of Existing (See Note 2, Sheet 3) Approach Slab

VIEW C-C

CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

 $\frac{1}{4}$ " $\pm \frac{1}{2}$ " Out of

Plumb (Max.) *

— @ Guardrail Post

- 5⁄8" Ø x 8" Post

Slope:

Depth respectively).

SECTION B-B TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB

(SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Varies

Bolts and Recessed Nuts

or "C" (See Roadway Plans)

Guardrail Post Assembly "A", "B"

Existing Curb Overhang

__ 5" Min.

drilled holes ($5\frac{1}{2}$ " Max. Depth)

 $2 \sim 1\frac{1}{4}$ " Ø x 1'-4" (1'-1" Min. Embed. Schemes 3 & 5) or $2 \sim 1\frac{1}{4}$ " Ø x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes $(1'-1\frac{1}{2}")$ or $5\frac{1}{2}"$ Max.

Embedment

 $\sim \frac{1}{8}$ " Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

Existing Wing Post

Existing Wing

(Type Varies)

For location of Section C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

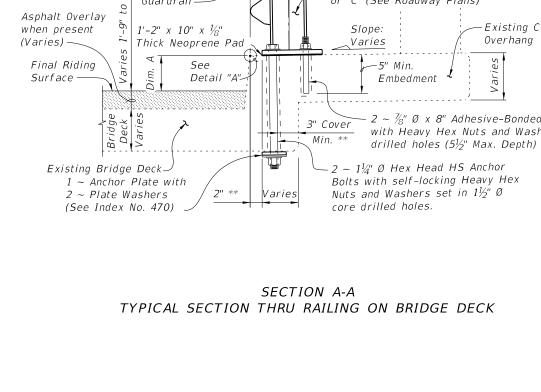
on Index 470, Sheet 3.

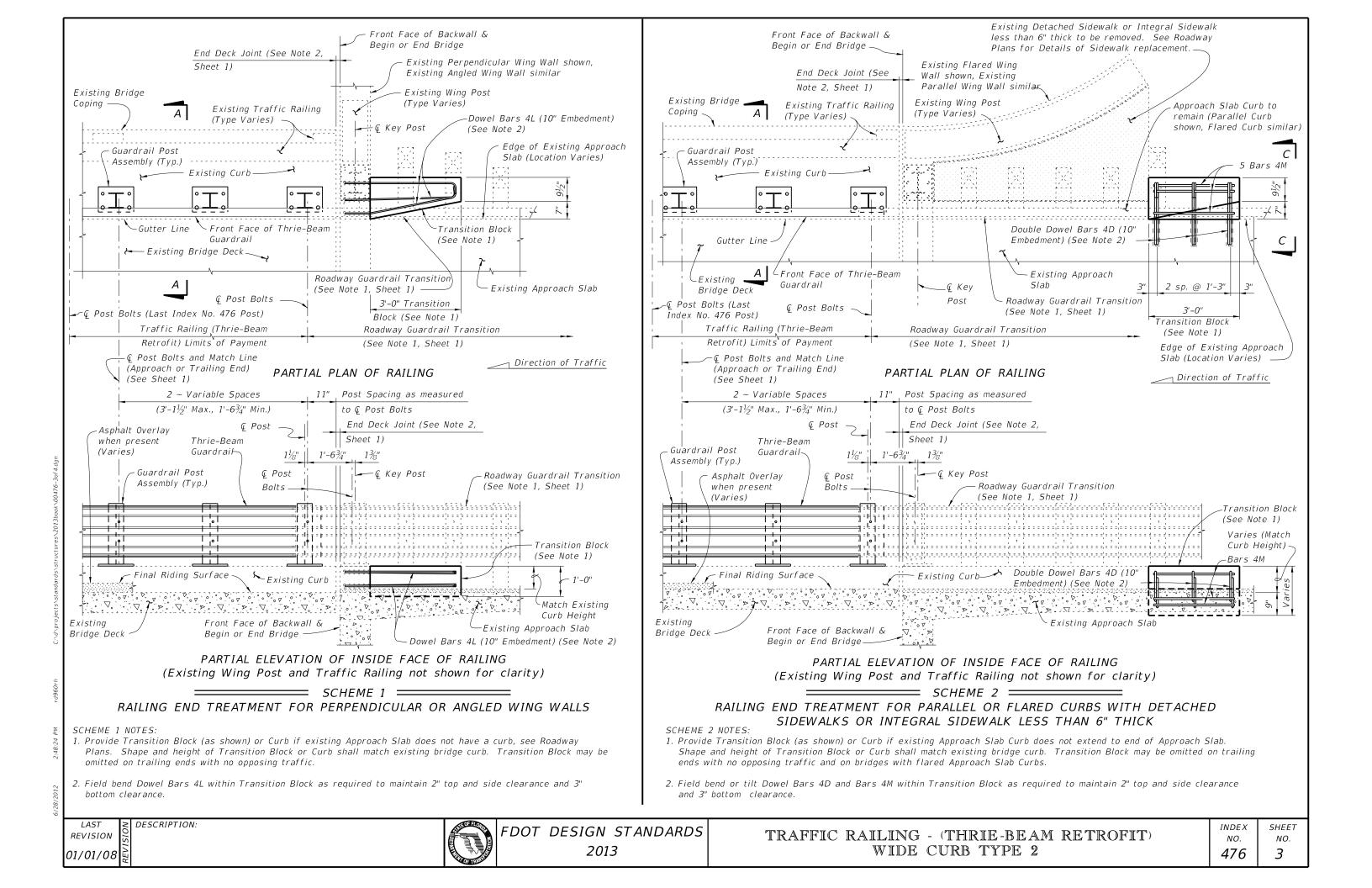
FDOT DESIGN STANDARDS 2013

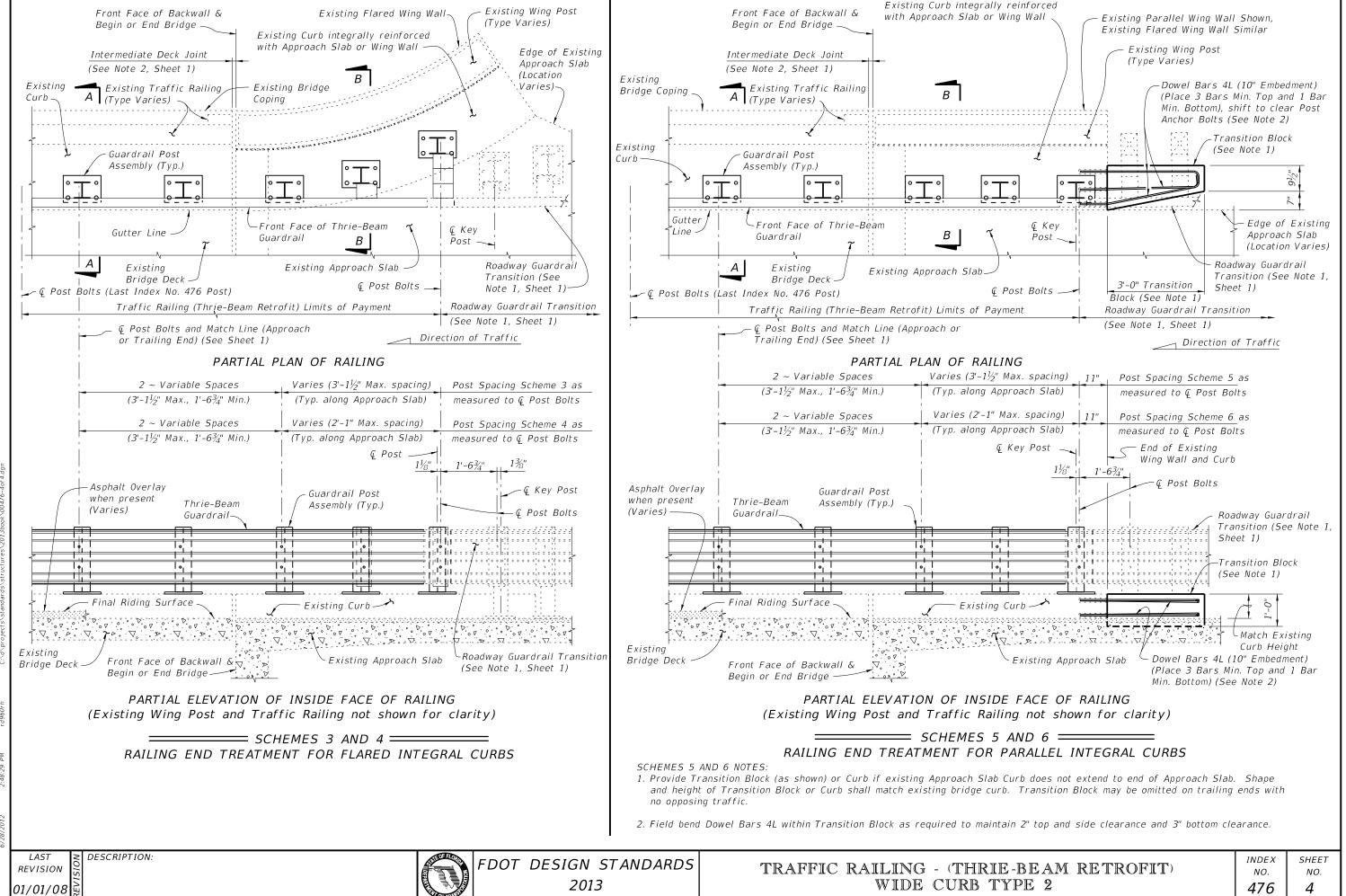
TRAFFIC RAILING - (THRIE-BEAM RETROFIT) WIDE CURB TYPE 2

INDEX NO. 476

SHEET NO. 2







CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment).

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

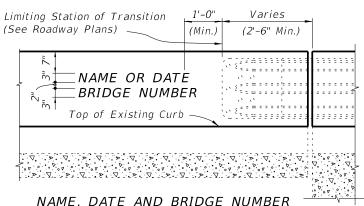
NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or 3/8" V-Grooves. V-Grooves shall be formed by preformed letters and figures.

ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The Department will determine the vertical Datum information for the marker.

SURFACE FINISH: Unless otherwise shown in the Plans, place a Class 5 Applied Finish Coating on the top and sides of the Traffic Railing (Vertical Face Retrofit).

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table below. Reflector color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) include all materials and labor required to construct the railing. Incidental work as required for transition blocks, curbs, spread footings approaches, reflective railing markers (including installation) shall also be included under Traffic Railing (Vertical Face Retofit).

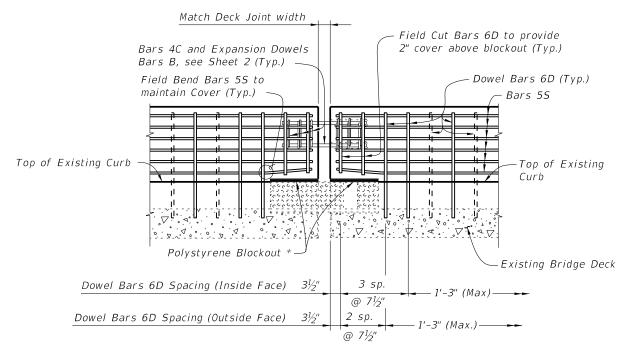


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ESTIMATED TRAFFIC RAILING QUANTITIES							
LINIT	QUANTITY						
UNIT	9" Curb	Increment					
CY/FT	0.064	0.003 per in. height					
LB/FT	13.27	0.10 per in. length					
	UNIT CY/FT	UNIT QUAN 9" Curb CY/FT 0.064					

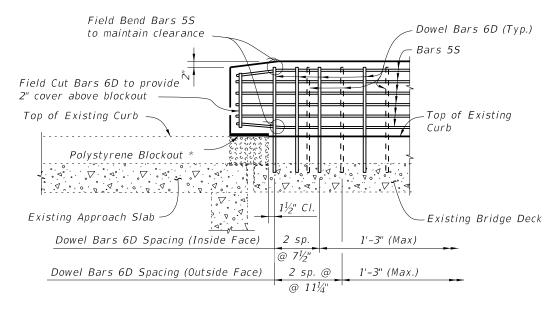
REFLECTIVE RAILING MARKER SPACING			
Distance – Edge of Travel Lane to Face of Railing	Spacing (Ft.)		
< 4'	40'		
4' to 8'	80'		
> than 8'	None Required		

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index No. 484, Sheet 4 for Spread Footing Approach Quantities.



PARTIAL ELEVATION OF RAILING SHOWING INTERIOR FINGER/SLIDING PLATE JOINT (Beam/Girder, Intermediate Bent or Pier not shown for clairty)

* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING SLIDING PLATE JOINT AT BEGIN OR END BRIDGE

(Scheme 1 shown, Schemes 2, 3 and 4 similar) (Guardrail Transition or continuation of Traffic Railing not show for clarity)

LAST REVISION 07/01/10

DESCRIPTION:



FDOT DESIGN STANDARDS 2013

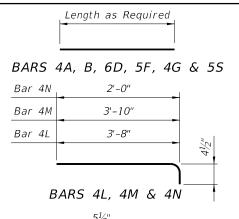
GENERAL NOTES & DETAILS

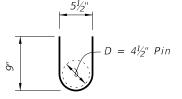
INDEX NO. 480

SHEET NO.

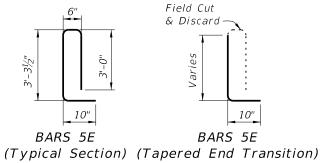
REINFORCING STEEL NOTES:

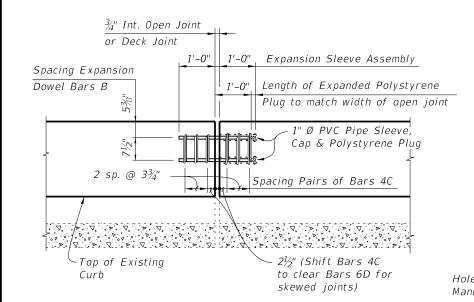
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.



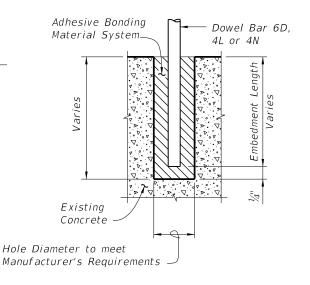


BARS 4C (12 required per open joint)





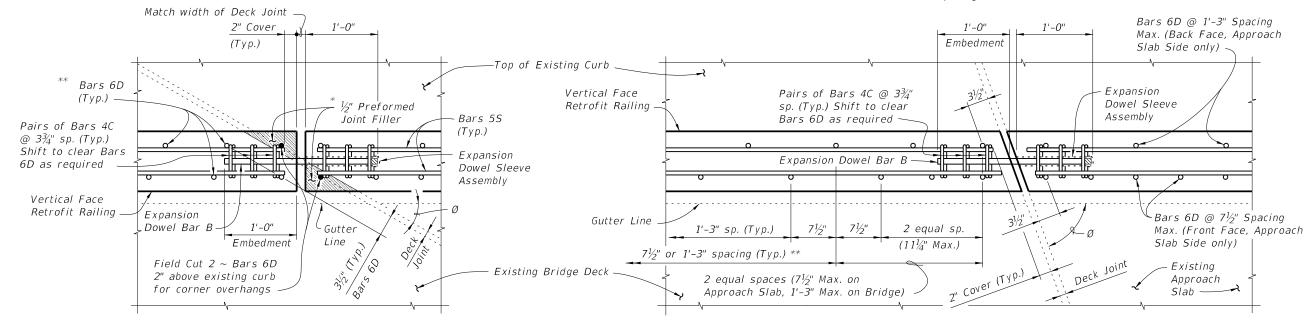
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



DOWEL DETAIL

Dowel Installation Notes:

- 1. Shift dowel holes to clear if the existing reinforcement is encountered.
- 2. See individual Standards Index Nos. 481 thru 484 for required embedment length of Bars 6D, 4L or 4N.
- * $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.
- ** See individual Standard Index Nos. 481 thru 484 for spacing of Bars 6D.



PARTIAL PLAN OF RAILING (SKEW ANGLE Ø LESS THAN 70°) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

SKEW DETAIL

PARTIAL PLAN OF RAILING (SKEW ANGLE $\emptyset = 70^{\circ}$ OR GREATER) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

DESCRIPTION: LAST REVISION



DOT DESIGN STANDARDS 2013

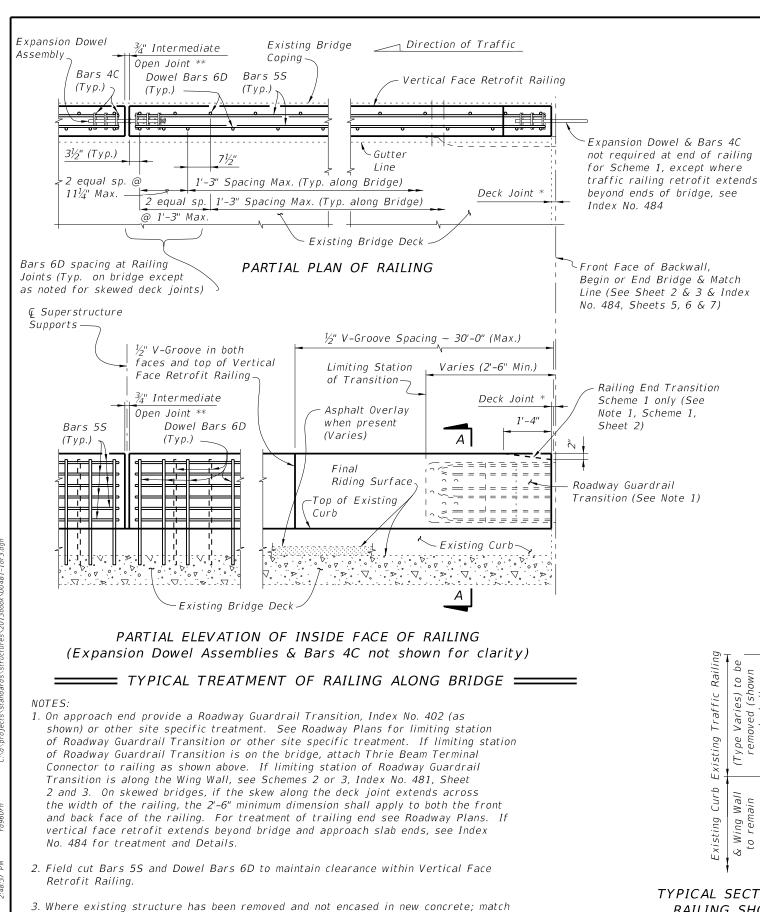
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) GENERAL NOTES & DETAILS

INDEX NO.

SHEET NO. 2

07/01/09

480



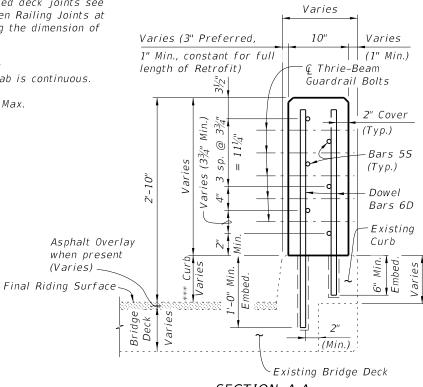
adjoining areas and finish flat by grouting or grinding as required. Exposed existing

reinforcing steel not encased in new concrete shall be burned off 1" below existing

* Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index No. 480. Provide open Railing Joints at Deck Expansion Joint locations matching the dimension of the Deck Joint.

** Provide 3/4" Intermediate Open Joints at: (1) - Superstructure supports where slab is continuous.

*** Curb heights vary from 5" Min. to 1'-2" Max.



SECTION A-A TYPICAL SECTION THRU RAILING ON CURB WITH CORBELS

Varies

CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagrams see Index No. 480.

Varies (3" Preferred, 10" Varies 1" Min., constant for full (1" Min.) length of Retrofit) ⊊ Thrie-Beam Guardrail Bolts 2" Cover (Typ.)Bars 5S (Typ.)Dowel Bars 6D -Existing Asphalt Overlay Curb when present (Varies) _ Final Riding Surface. 1'-0" Emb idge Br D Existing Bridge Deck

TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

SECTION A-A TYPICAL SECTION THRU RAILING ON FULL DEPTH CURB (BRIDGE SHOWN, WING WALL SIMILAR)

Curb Existing Wir to Ø

LAST 07/01/10



FDOT DESIGN STANDARDS 2013

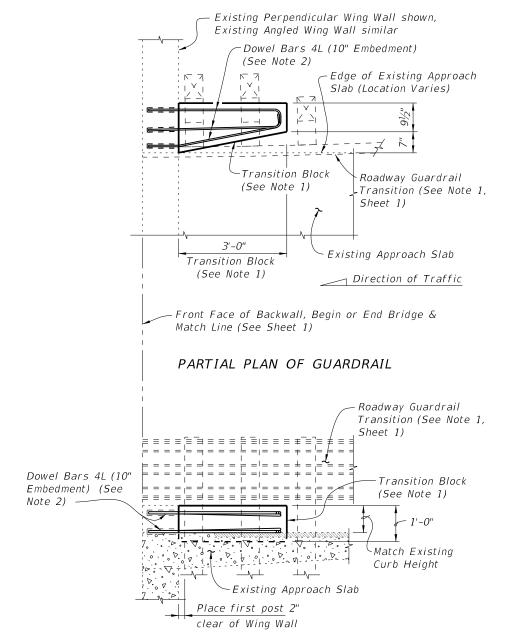
TRAFFIC RAILING - (VERTICAL FACE RETROFIT) NARROW CURB

INDEX NO. 481

SHEET NO.

DESCRIPTION: REVISION

concrete and grouted over.



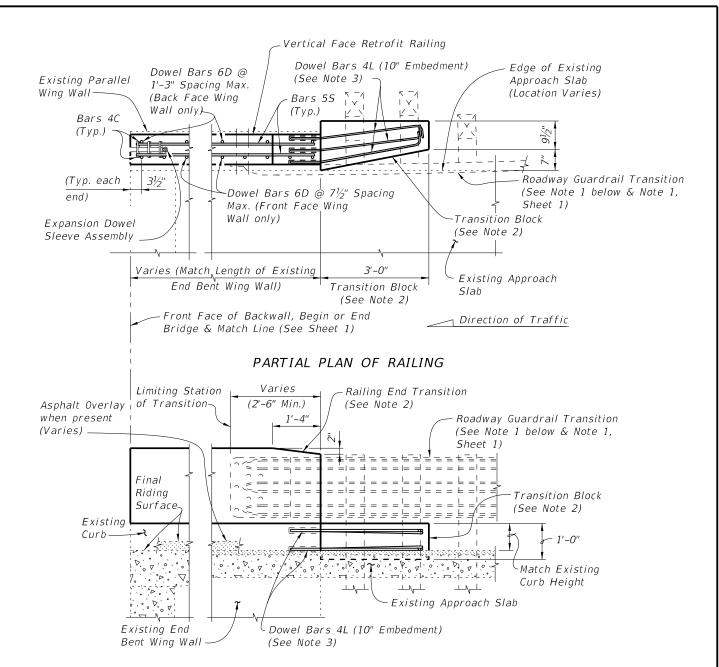
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

_____ SCHEME 1 _____ RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

DESCRIPTION:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

_____ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL WING WALLS

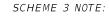
SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Index No. 481, Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom

DESCRIPTION:



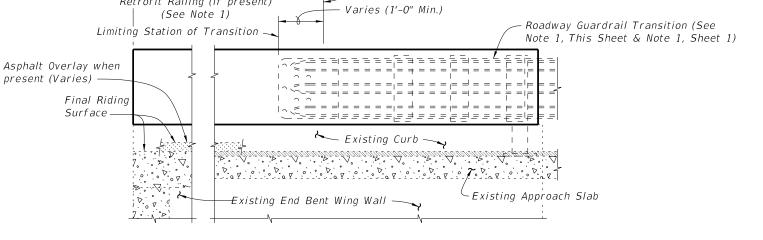




TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

NARROW CURB

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see



-Begin Flared Portion of Wing Wall

and Vertical Face Retrofit Railing

Existing Flared

Wing Wall

Bars 5S (Field

Bend) (Typ.)

Dowel Bars 6D @ 7½" Spacing

Existing Approach Slab ——

Front Face of Backwall, Begin or End Bridge &

Match Line (See Sheet 1)

Parallel Portion of Vertical Face Retrofit Railing (if present)

Max. (Front Face Wing Wall only)

Varies (Match Length of Existing End Bent Wing Wall)

PARTIAL PLAN OF RAILING

PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

Dowel Bars 6D @ 1'-3" Spacing

Max. (Back Face Wing Wall only)

(Typ. each

end)

Gutter Line

Vertical Face Retrofit Railing

Bars 4C (Typ.)

Expansion Dowel

Sleeve Assembly

RAILING END TREATMENT FOR FLARED WING WALLS

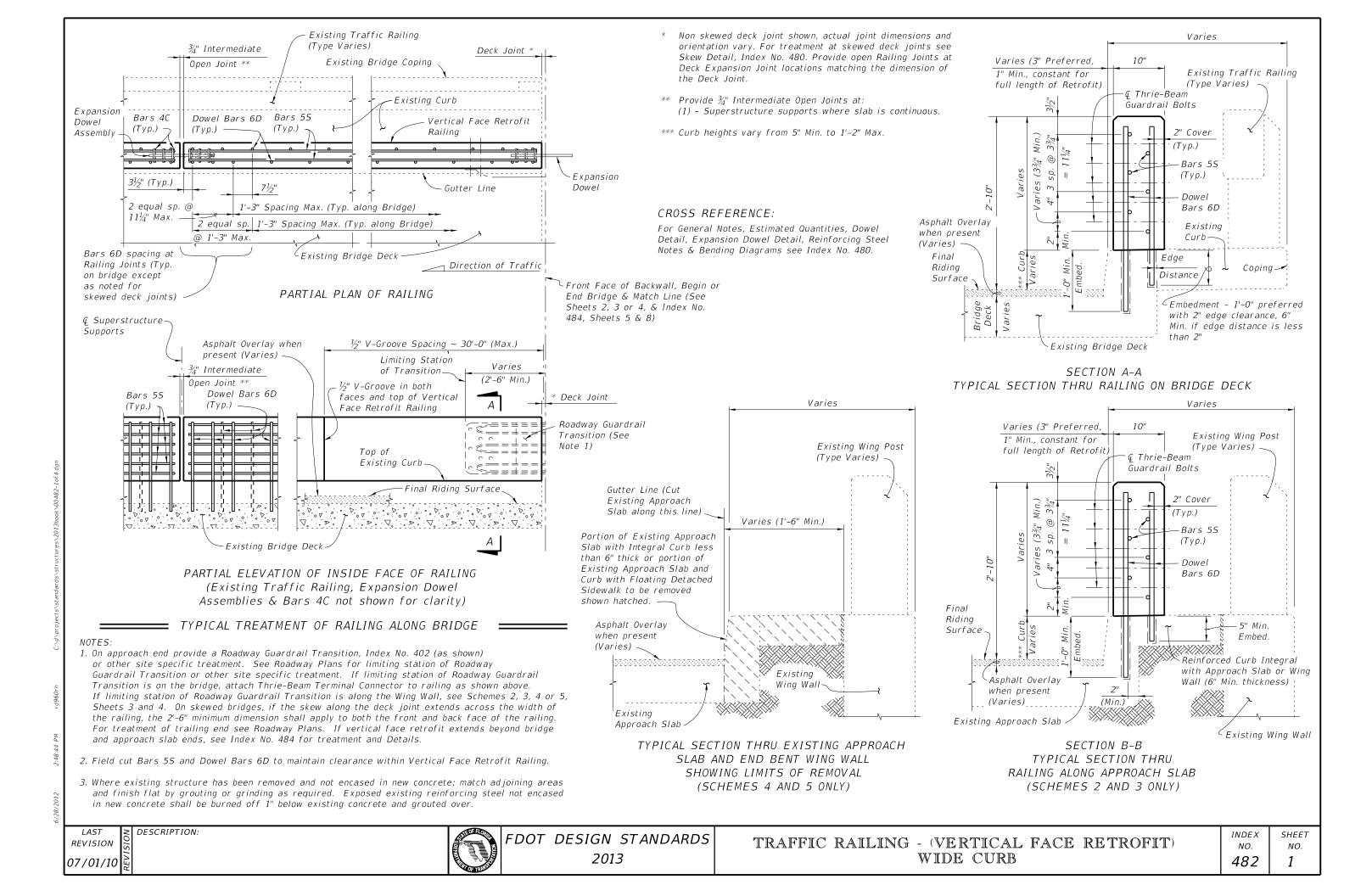
> INDEX NO.

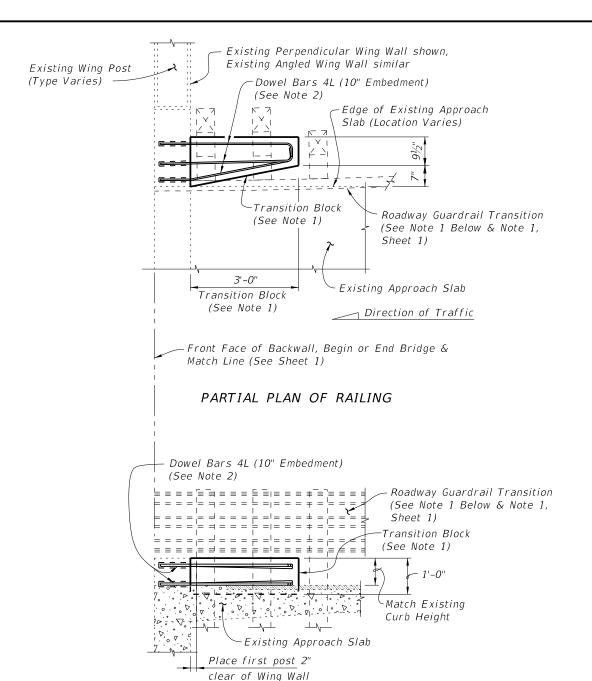
Edge of Existing Approach Slab (Location Varies)

Roadway Guardrail Transition

____ Direction of Traffic

(See Note 1, This Sheet & Note 1, Sheet 1)





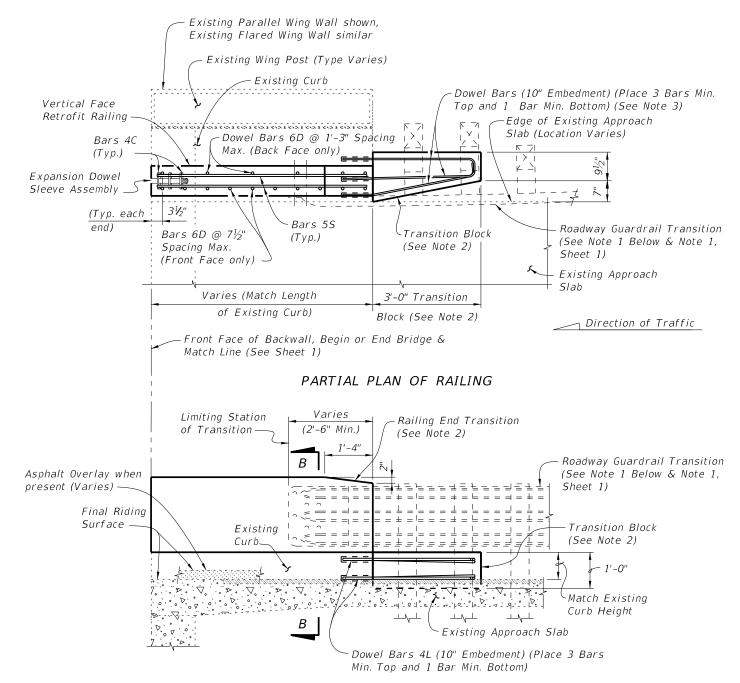
PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

SCHEME 1 RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

DESCRIPTION:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3"
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



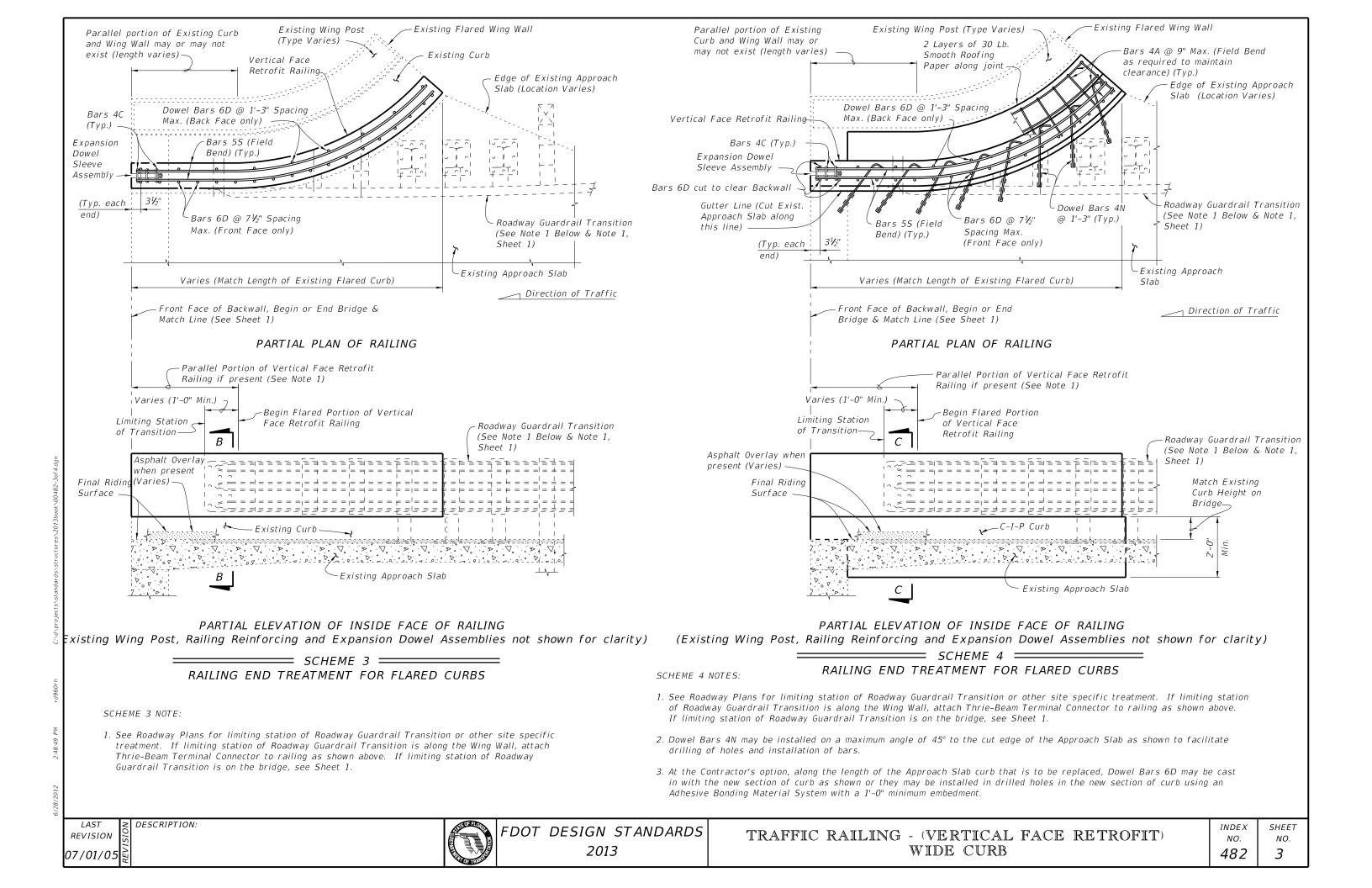
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

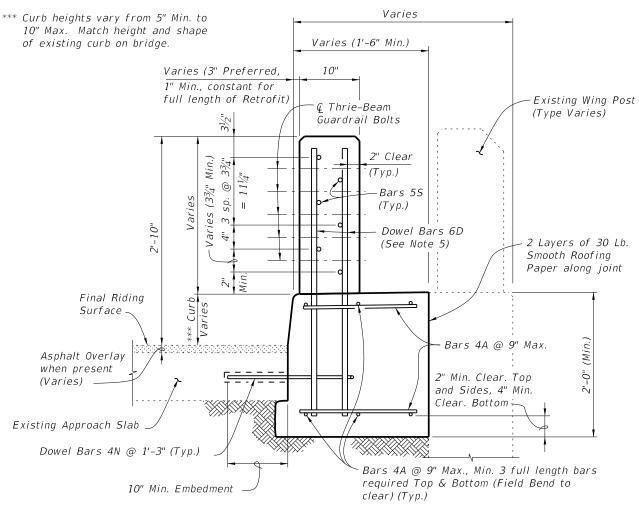
> _____ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL CURBS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.







SECTION C-C TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

SCHEME 5 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge,
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.

LAST DESCRIPTION: REVISION 07/01/05

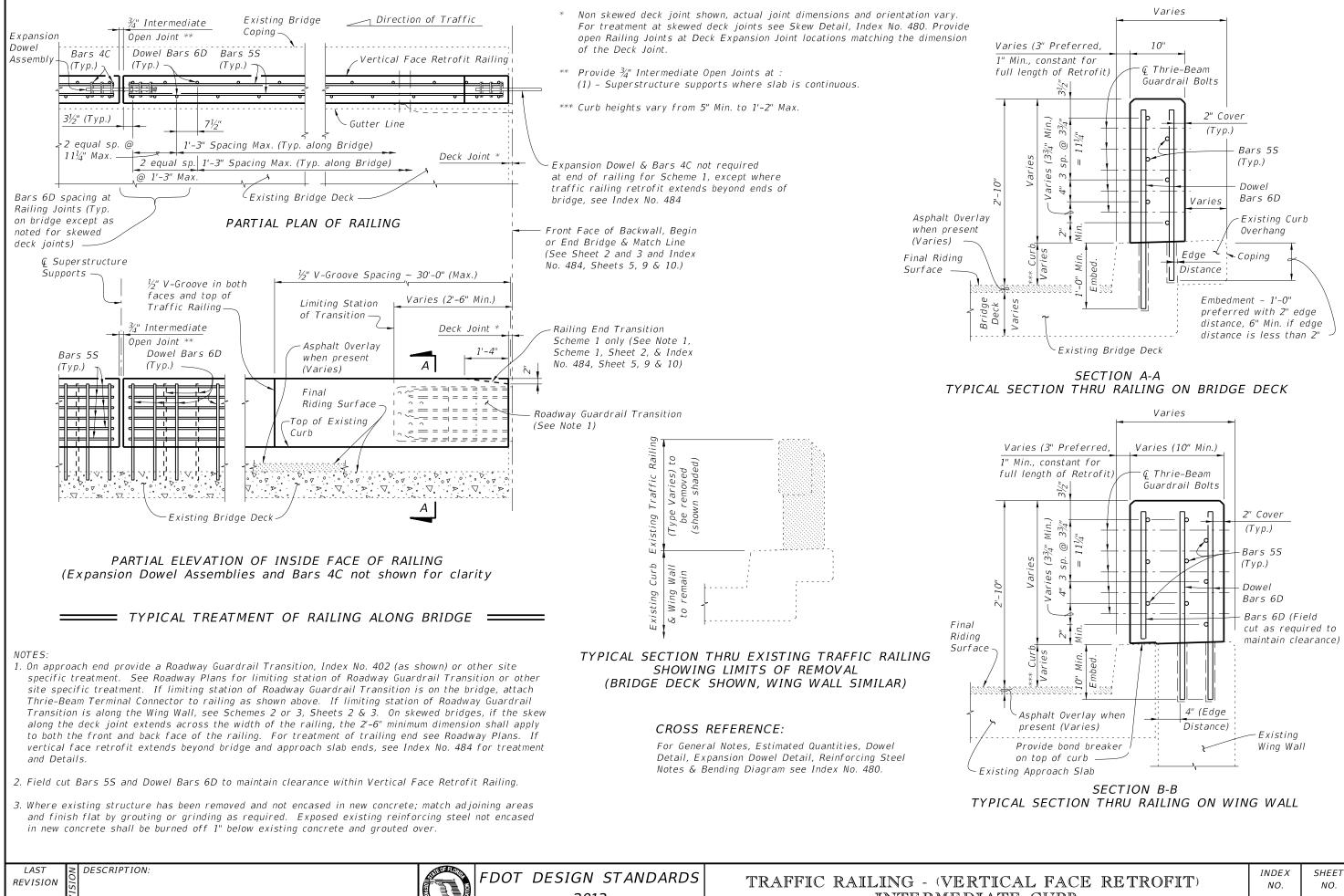


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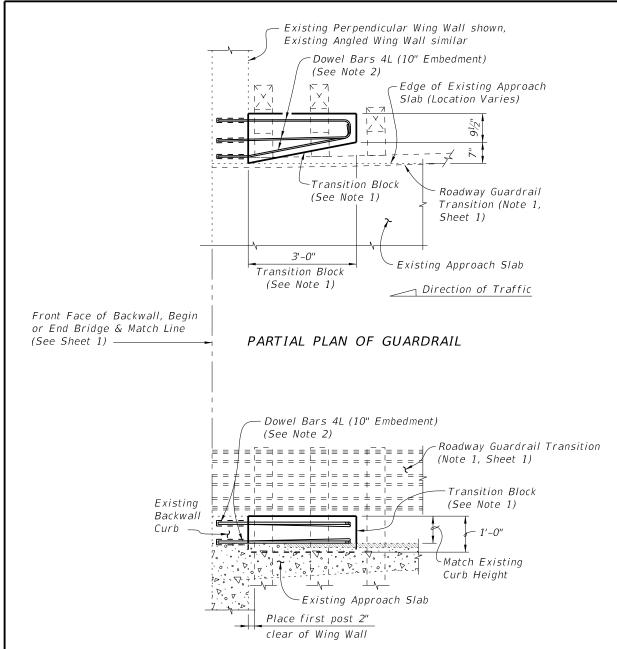
INDEX SHEET NO. 482

NO. 4

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) WIDE CURB





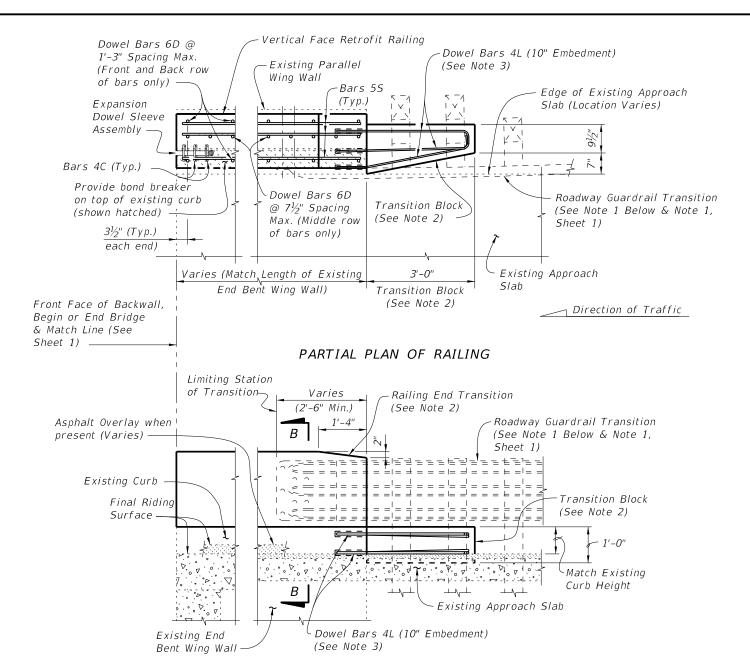


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

______ SCHEME 1 _____ RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

______ SCHEME 2 _____ RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

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2013

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" mminimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" hottom clearance.

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

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= SCHEME 3 ===== RAILING END TREATMENT FOR FLARED WING WALLS

LAST REVISION 07/01/07

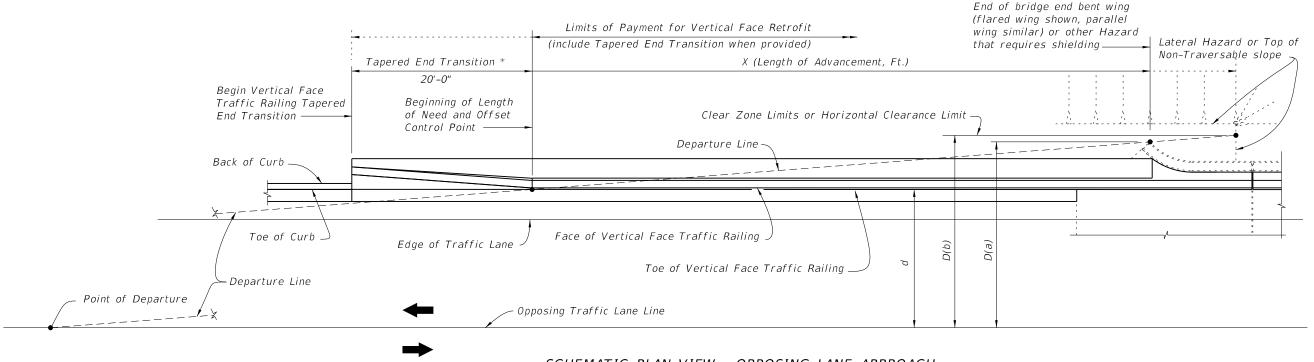
DESCRIPTION:



or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector

to railing as shown above. If limiting station of Roadway Guardrail

Transition is on the bridge, see Sheet 1.



SCHEMATIC PLAN VIEW - OPPOSING LANE APPROACH

Design Speed (mph)	Length of Advancement, Ft. (X)			
≤ 40	= 16 (D-d)			

07/01/09

- 1. The minimum length of advancement for both near lane and opposing lane approaches is 20'.
- 2. For Design Speeds greater than 40 mph the Tapered End Transition is not permitted. See Index No. 400 for length of Advancement of guardrail or other project specific end treatments.

= LENGTH OF ADVANCEMENT - TAPERED END TRANSITION (40 MPH OR LESS) =====

DESIGN NOTES:

The Tapered End Transition should only be used when space is limited which precludes the use of a quardrail end treatment or crash cushion.

- D = Distance in feet from near edge of near approach traffic lane to either: (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance; (b) the clear zone or horizontal clearance outer limits, when hazard extends to, or goes beyond the clear zone or horizontal clearance limits.
 - For left side hazards on two way undivided facilities, "D" is measured from the inside edge of the near approach traffic lane as shown above.
- d = Distance in feet from near edge of near approach traffic lane to face of traffic railing (at offset control point). For left side hazards on two-way undivided facilities "d" is measured from the inside edge of the nearest opposing traffic lane as shown above.

CROSS REFERENCES:

For General Notes, Dowel Details, Expansion Dowel Details, Reinforcing Steel Notes and Reinforcing Steel Bending Diagram see Index No. 480.

DESCRIPTION: LAST REVISION

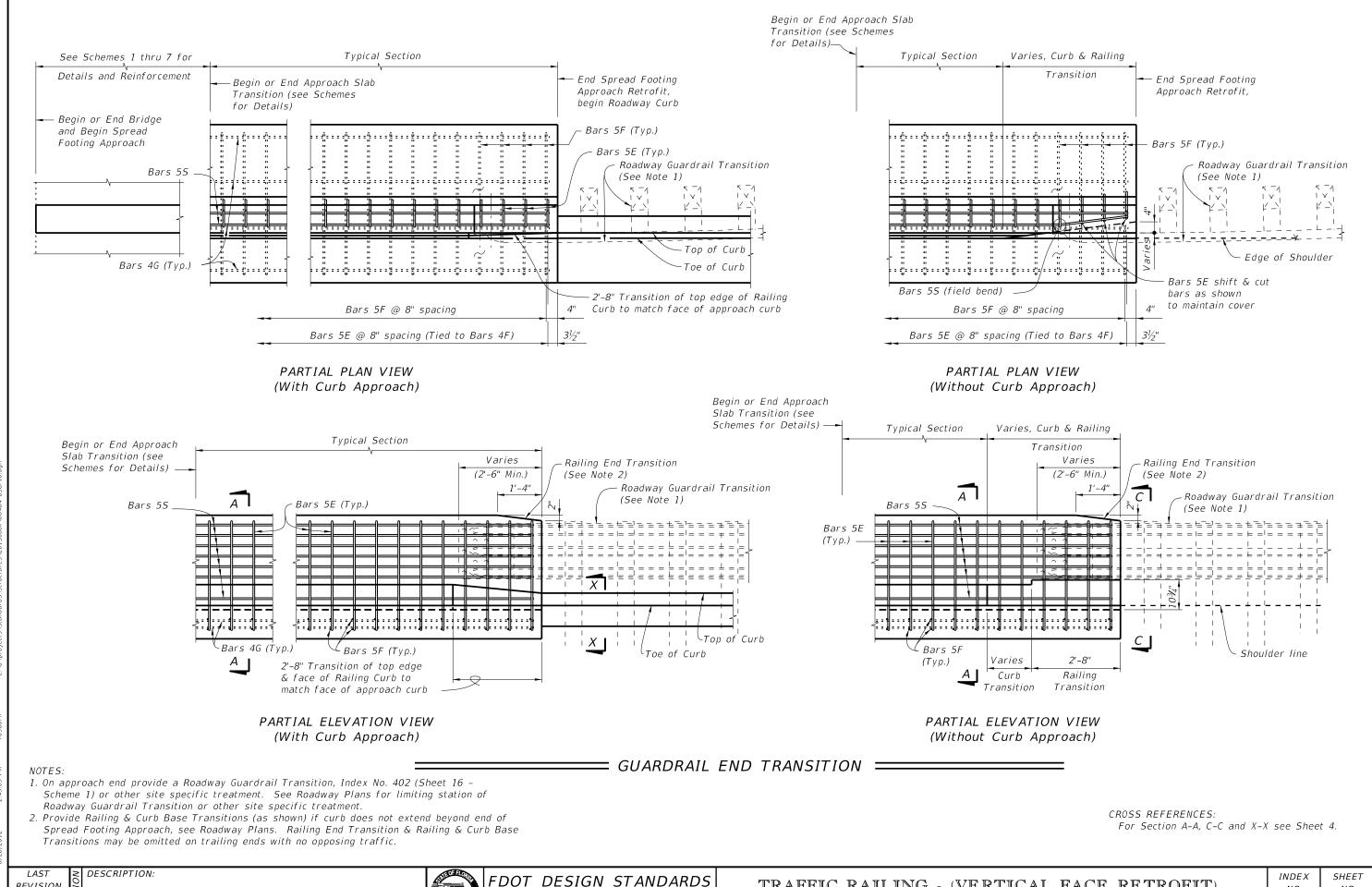


DOT DESIGN STANDARDS 2013

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH

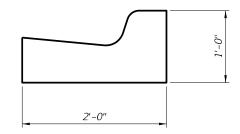
SHEET *INDEX* NO. 484

NO.



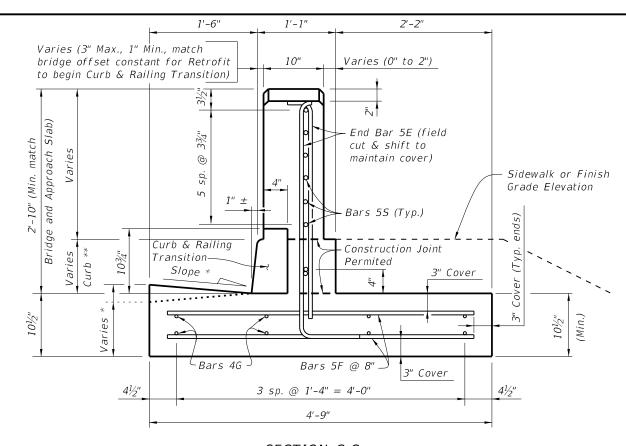
REVISION 07/01/09

NOTE: Quantities are based on a 9" curb, no curb cross slope.

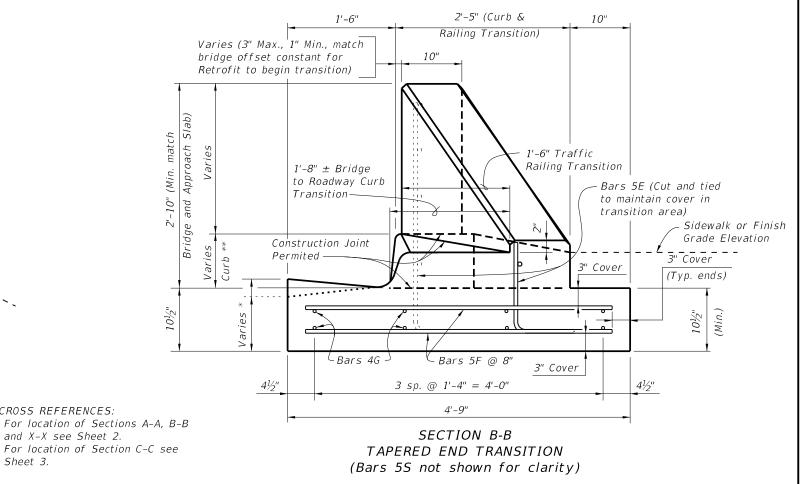


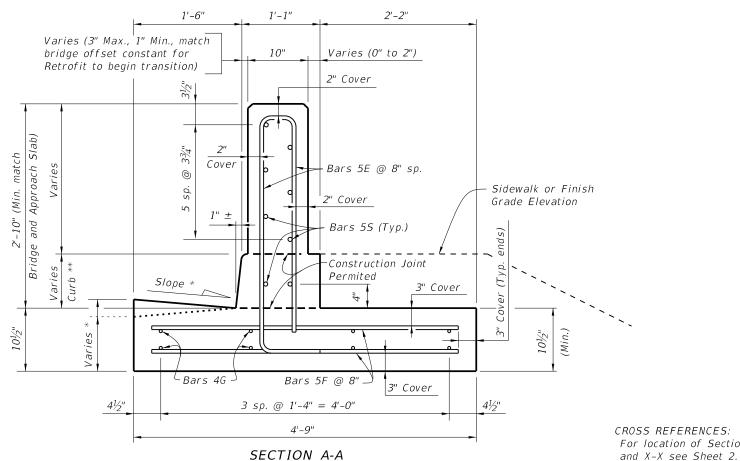
SECTION X-X (TYPICAL CURB, TYPE VARIES, TYPE F SHOWN) (See Index No. 300 and Plans for Details)

- * Match Cross Slope of high side and low side at begin or end bridge or approach slab.
- ** Match curb height of adjacent bridge and approach slab. Adjust height in Transition area to match adjoining Roadway curb.



SECTION C-C (GUARDRAIL END TRANSITION)

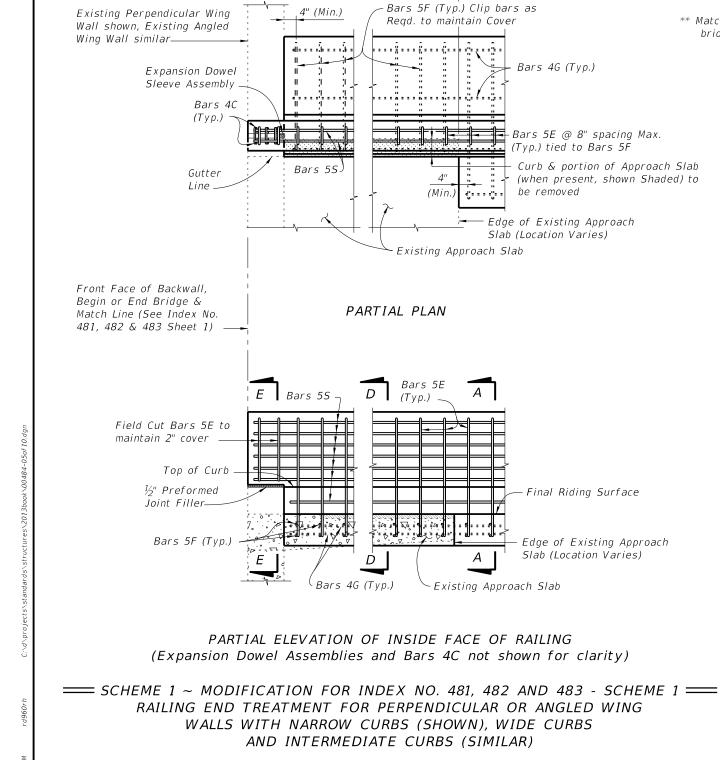




SECTION A-A TYPICAL SECTION (9" Curb shown, 6" Curb similar)

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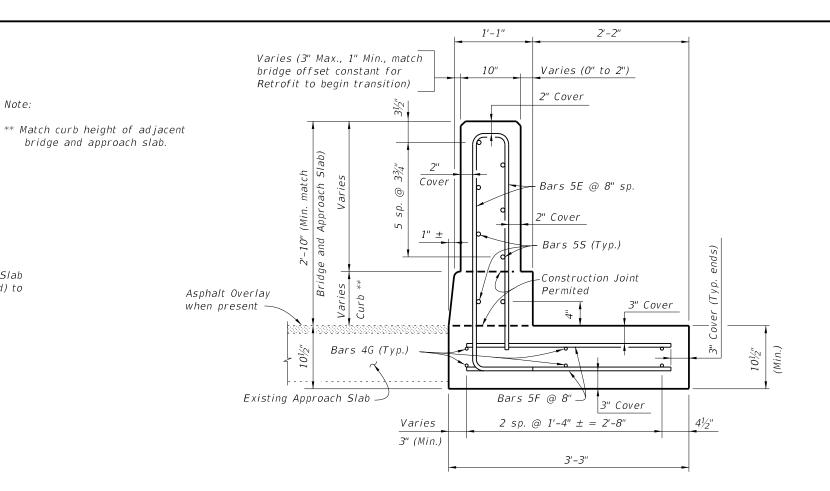
Sheet 3.



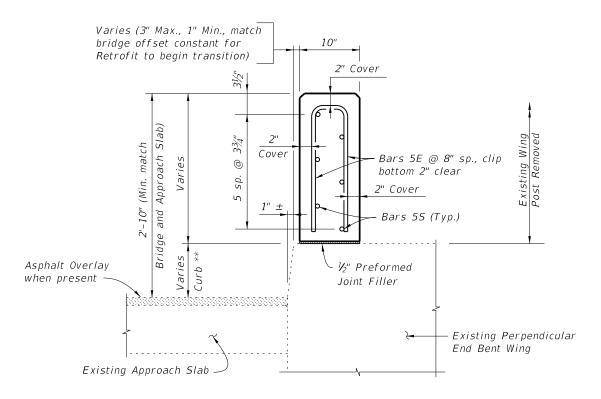
Approach Slab Transition

Typical Section

Note:



SECTION D-D



SECTION E-E (NARROW CURB SHOWN, WIDE AND INTERMEDIATE CURBS SIMILAR)

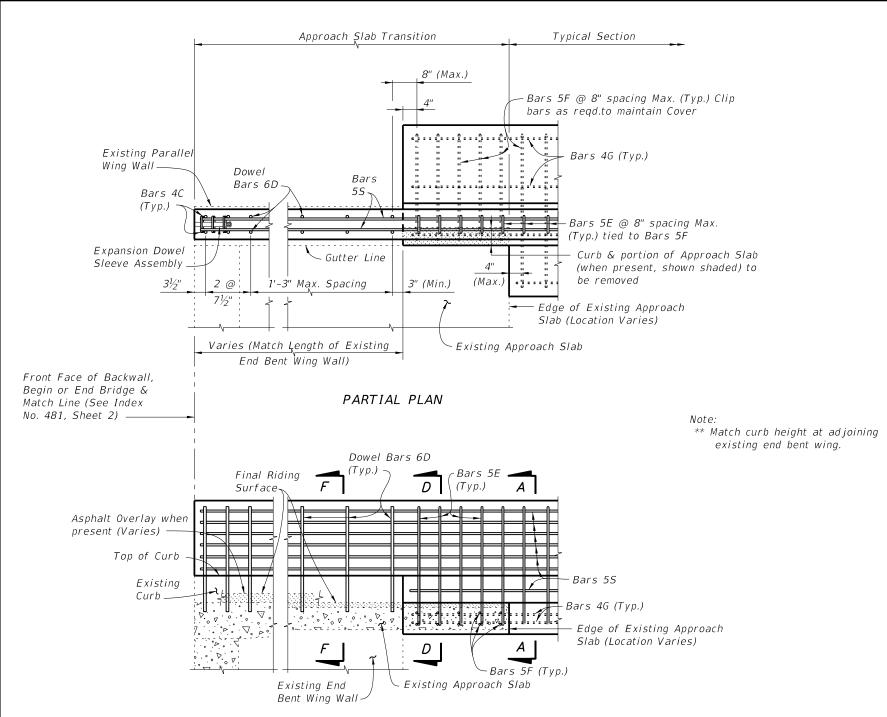
07/01/09

CROSS REFERENCE:

see Index 480.

DESCRIPTION:

For Section A-A see Sheet 4. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details



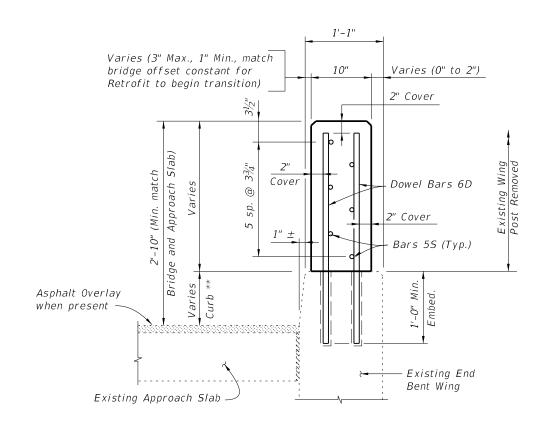
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

SCHEME 2 ~ MODIFICATION FOR INDEX NO. 481 - SCHEME 2 ====== RAILING END TREATMENT FOR PARALLEL WING WALLS WITH NARROW CURBS

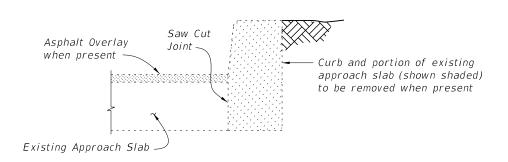
NOTES:

DESCRIPTION:

1. Remove existing concrete along saw cut joints. Existing reinforcing steel may be cut at joint or extended into new concrete. Exposed existing reinforcing not encased in new concrete shall be removed 1" below existing concrete surface and grouted over.



SECTION F-F



SECTION THRU EXISTING CURB AND APPROACH SLAB TO BE REMOVED (Free Standing Curb Similar)

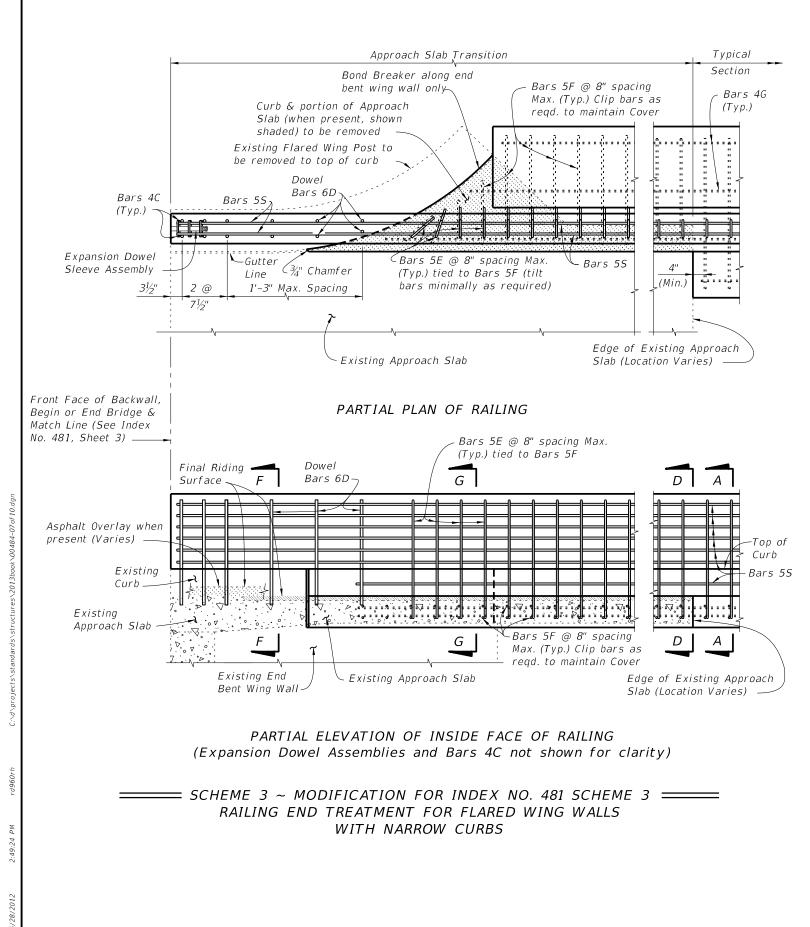
CROSS REFERENCES:

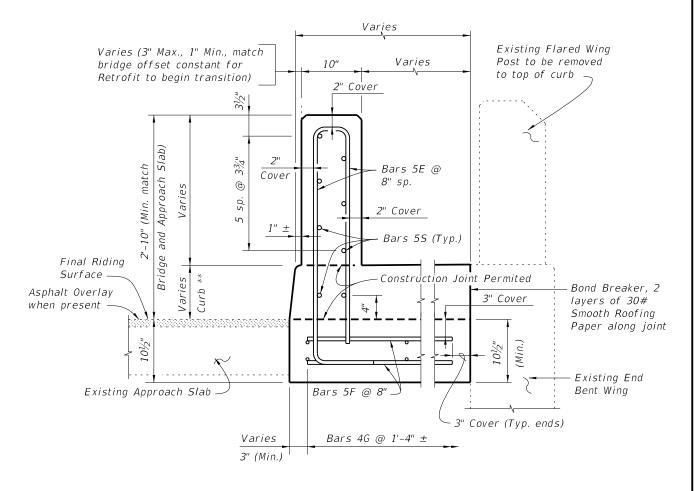
For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 480.

REVISION 07/01/09

LAST

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)





SECTION G-G

Note:

** Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section F-F see Sheet 6.

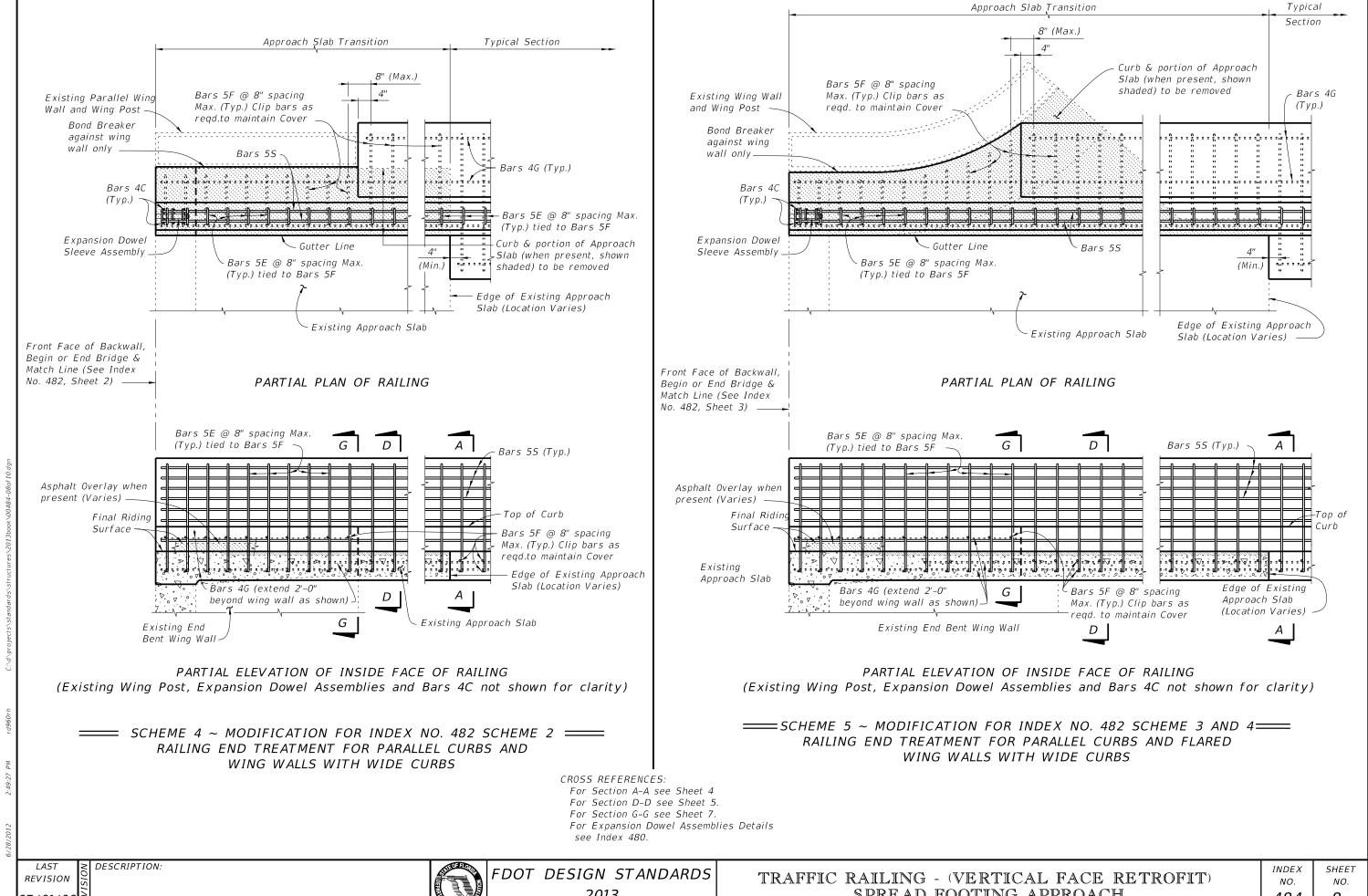
For Expansion Dowel Assemblies Details and

placement of Dowel Bars 6D see Index 480.

LAST REVISION 07/01/09

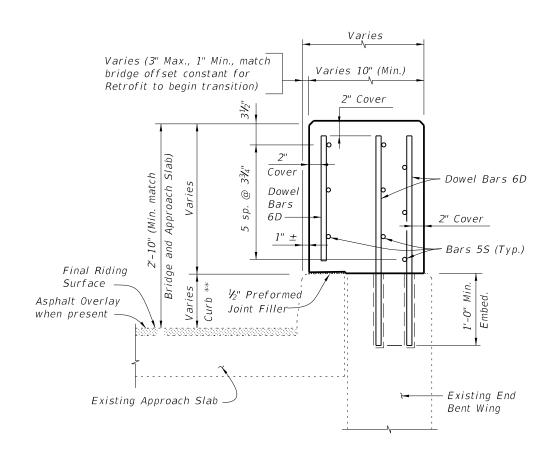
DESCRIPTION:

FDOT DESIGN STANDARDS 2013



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

── SCHEME 6 ~ MODIFICATION FOR INDEX NO. 483 SCHEME 2 ───── RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH INTERMEDIATE CURBS

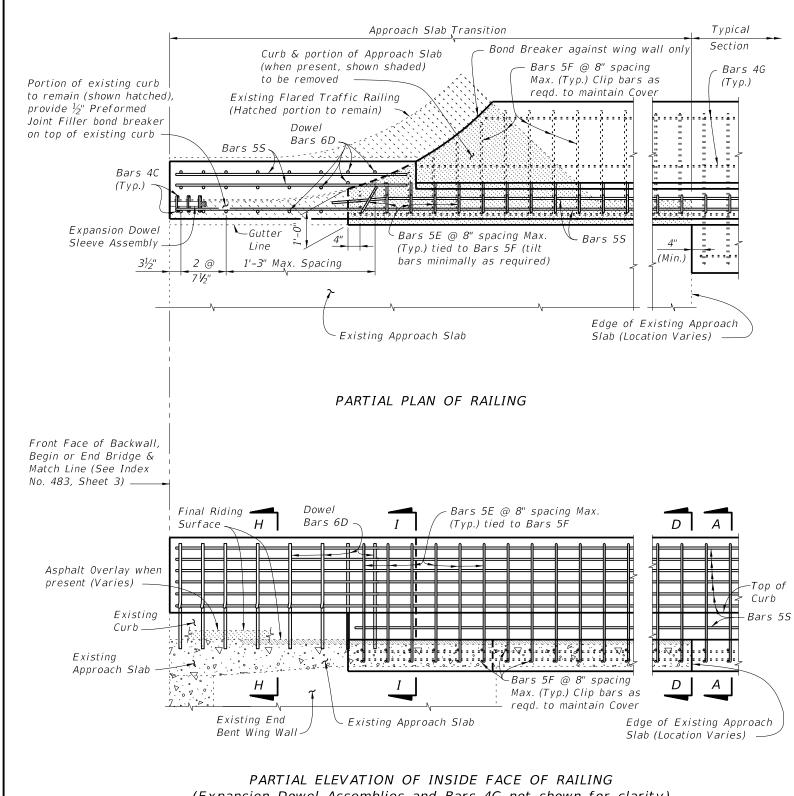


SECTION H-H

Note:

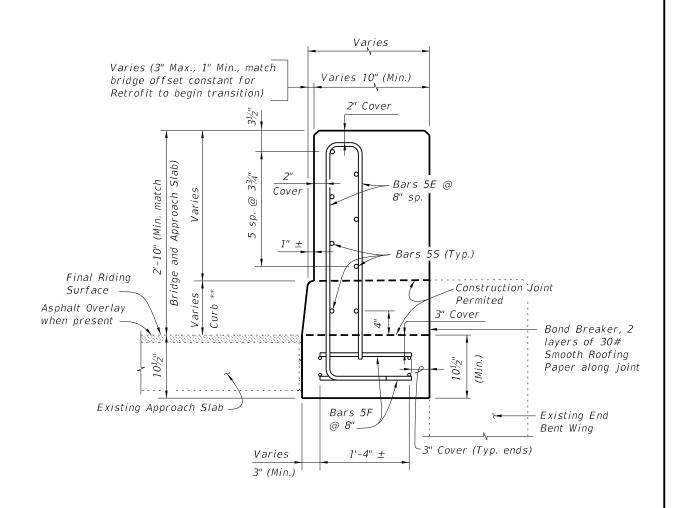
** Match curb height at adjoining existing end bent wing.

> CROSS REFERENCES: For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 480.



(Expansion Dowel Assemblies and Bars 4C not shown for clarity)

=== SCHEME 7 ~ MODIFICATION FOR INDEX NO. 483 SCHEME 3 ====== RAILING END TREATMENT FOR PARALLEL CURBS AND FLARED WING WALLS WITH INTERMEDIATE CURBS



SECTION I-I

Note: ** Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4. For Section D-D see Sheet 5. For Section H-H see Sheet 9. For Expansion Dowel Assemblies and placement of Dowel Bars 6D Details see Index 480.

LAST REVISION 07/01/09

DESCRIPTION:



FDOT DESIGN STANDARDS 2013