							STANDARD CRITER	I A					
CLASS	17PE	APPLICATION DESCRIPTION	INDEX NO.	PERMITTIVITY (sec ⁻¹)	AOS SIEVE#	MIN. GRAB TENSILE	MIN. SEWN STRENGTH	MIN. PUNCTURE	MIN. TRAPEZOIDAL TEAR	MIN. WIDE WIDTH TENSILE	RESIS	IV STANCE Mowed)	COMMENTS
	ì			, ,		STRENGTH (lb)	(Ib/in)	(Ib)	(Ib)	STRENGTH (lb/in)	%	Time (Hrs.)	
	D-1	Revetment (Special)		(See D-2)	(See D-2)	315	7.2	113	113		50	500	Woven Monofilament Geotextiles only (Elongation <50%) Provide 12" thick bedding stone layer.
		Revetment (Standard)		OV COLL DAGGING	OV COLL DAGGING	Woven	Woven Monofilament	Woven	Woven				Woven Geotextiles only. No Slit Film Geotextiles allowed. Provide 12" thick bedding stone layer for revetment
	D-2	Articulating Block****		% SOIL PASSING No. 200 SIEVE <15% 0.7	% SOIL PASSING No. 200 SIEVE <15% 40	Monofilament 248 Other Geotextiles:	5.7 Other Geotextiles:	Monofilament 90 Other Geotextiles:	Monofilament 57 Other Geotextiles:		50	500	(standard). The bedding layer may be omitted if a D-1 fabric is used with revetment (standard).
	D-2	Gabions	281	15% to 50% 0.2 >50% 0.1	15% to 50% 60 >50% 70*	Elongation <50% 315	Elongation <50% 6.9	Elongation <50% 113	Elongation <50% 113		50 50	300	****Bedding Stone not required for Articulating Block. *For cohesive soils with a plasticity index >7, maximum
		Rock, Rubble, Broken Concrete		1 3070	70	≥50% 203		≥50% 79	≥50% 79				average role value for AOS is number 50 sieve.
DRAINAGE (D)		Underdrain ***	286	% SOIL PASSING % SOIL PASS	% SOIL PASSING								No woven slit film fabrics allowed.
		French Drain	285	No. 200 SIEVE	No. 200 SIEVE	Elongation	Elongation	Elongation	Elongation				*For cohesive soils with a plasticity index >7, maximum
	D-3	Sheet Piling Filter		<15% 0.5	<15% 40	<50% 248	<50% 5.7	<50% 90	<50% 90**		50	500	average role value for AOS is number 50 sieve. **Required Trapezoidal tear for woven monofilament is 250.
		Filter Fabric Jacket (Culvert) Concrete Pavement Subdrainage	280 287	15% to 50% 0.2 >50% 0.1	15% to 50% 60 >50% 70*	≥50% 158	≥50% 3.6	≥50% 57	≥50% 57				***See Index No. 286 for the permittivity and AOS values of the internal filter fabric of Type V Underdrain.
	D-4	Slope Pavement (Sand-Cement) Ditch Pavement (Sand-Cement)	281	0.5	40	180	4.2	50	35		50	500	Non-woven, needle-punch only. Elongation ≥50%
		Mechanical Stabilized Retaining Wall	1 201										
	D-5	Cast-In-Place Retaining Wall		0.5	40	90	2.1	50	40		50	500	
	D-6	Slope Pavement (Concrete) Ditch Pavement (Concrete)	281	0.5	40	180	4.2	50	35		50	500	Non-woven, needle-punch only. Elongation ≥50%
	E-1	Staked Silt Fence		0.05	NA	90	2.1	NA	35		80	500	Min. Filtration Efficiency of 75% & min. flow rate of 0.3 gal.
	E-2	Wind Screen		0.05	NA	90	2.1	NA	NA		80	150	
EROSION (E)	E-3	Plastic Erosion Mat (Turf Reinforcement Mat)(Type 1)		NA	NA	NA	NA	NA	NA	12 x 6	80	500	Use where design shear stress is ≤2.1 psf
	E-4	Plastic Erosion Mat (Turf Reinforcement Mat) (Type 2)		NA	NA	NA	NA	NA	NA	23 x 12	80	500	Use where design shear stress is ≤3.6 psf
	E-5	Plastic Erosion Mat (Turf Reinforcement Mat) (Type 3)		NA	NA	NA	NA	NA	NA	46 x 23	80	500	Use where design shear stress is ≤5.0 psf

(1) Type refers to FDOT class and application.

TABLE I

Test	Unit	Test Method
Permittivity AOS Elongation	sec-¹ US Sieve No. %	ASTM-D-4491 ASTM-D-4751 ASTM-D-4632
Grab Tensile Strength	1b	ASTM-D-4632
Wide Width Tensile Strength	lb/in	ASTM-D-4595
Maximum Design Velocity	fps	See Design Note 3
Sewn Strength	lb/in	ASTM-D-4884
Puncture	Ib	ASTM-D-4833
Trapezoidal Tear	1b	ASTM-D-4533
Ultraviolet Resistance	% Retained In Strength	ASTM-D-4355
Filtration Efficiency	%	ASTM-D-5141
Flow Rate	gal³/min	ASTM-D-5141

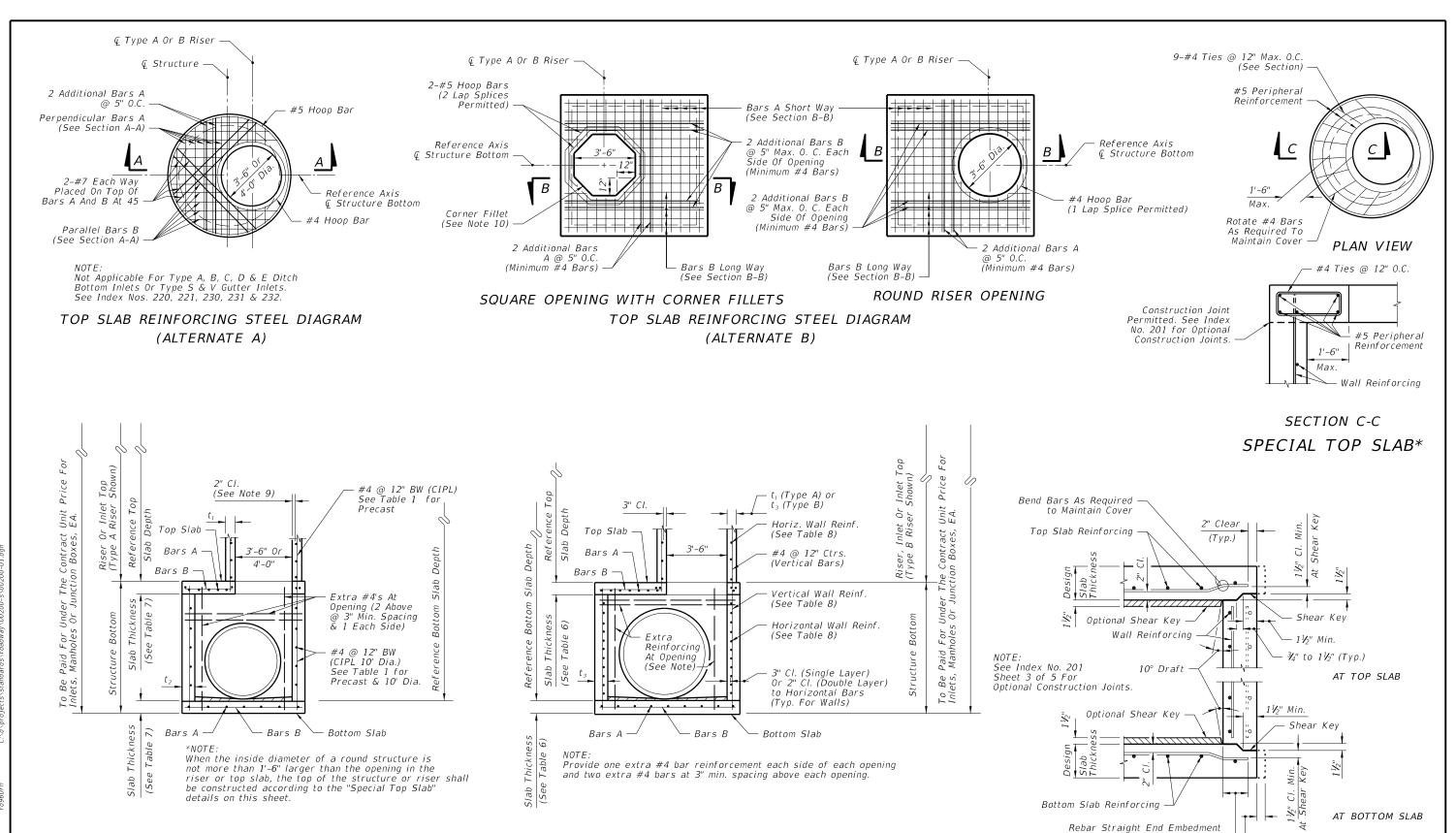
GENERAL NOTES

- 1. Specifications for geotextiles are in Section 985. Physical criteria for each application is provided by this standard, in conjunction with those sections.
- 2. All values except AOS are MINIMUM AVERAGE ROLL values in the weakest principal direction. Values for AOS are MAXIMUM AVERAGE ROLL values.
- 3. Test soil or fill material adjacent to the geotextile for gradation to select values for permittivity and AOS.
- 4. Unless specifically restricted in COMMENTS column, any type of material meeting specification 985 may be used.
- 5. Wide width tensile strength is expressed in units of measure of Ib./in., in machine direction and cross direction, as MD x CD.
- 6. The Manufacturer shall provide results in English Units.

DESIGN NOTES

- 1. The Designer shall review this criteria and adjust the values as necessary to satisfy project requirements. These adjustments shall be called for in the plans or contained in the project special provisions.
- 2. UV Resistance: The value represents the percent minimum textile strength retained (ASTM-D-4632) after weathering per ASTM-D-4355 for the test period (hours).
- 3. Shear stress limits for plastic erosion mats determined by 30 minutes sustained flow in unvegetated state as determined by tests performed by Utah State University, Texas Transportation Institute or an independent testing laboratory approved by the State Drainage Engineer.

07/01/12



SECTION A-A
(ALTERNATE A)

SECTION B-B
(ALTERNATE B)

TYPICAL SLAB TO WALL DETAILS
FOR PRECAST STRUCTURES

4" Min Revond Inside Face of

Structure Wall For All Bar Sizes _

LAST REVISION 01/01/10

DESCRIPTION:



Extend Top and Bottom Slabs To Achieve Minimum Rebar

Embedment Beyond Inside Face

ROUND STRUCTURE BOTTOMS (ALTERNATE A) & ROUND RISERS- TABLE 1 Wall Thickness (t₁ & t₂) and Vertical & Horizontal Areas of Reinforcement (A₅)

			,					()			
		Cast-In-Place Items			Precast Items						
	 Structure/Riser	Clas	ss II Cor	ncrete	Clas	ss II Cor	ncrete	ASTM C478			
Туре	Diameter (ft)	t ₁	t ₂	As	t ₁	t ₂	As	tı or t2	A2 ***		
		Riser (in.)	Bottom (in.)	(in.²/ft.)	Riser (in.)	Bottom (in.)	(in.²/ft.)	(in.)	(in.²/ft.)		
Р	3'-6"	6	8	0.20	6	8	0.20	4**	0.105		
Р	4'-0"	6	8	0.20	6	8	0.20	5**	0.120		
J	5'-0"	_	8	0.20	_	8	0.20	6**	0.150		
J	6'-0"	-	8	0.20	-	8	0.20	6	0.180		
J	7'-0"	_	8	0.20	_	8	0.20	7	0.210		
J	8'-0"	_	8	0.20	_	8	0.20	8	0.240		
J	10'-0"	_	10	0.40##	_	10	0.40##	10	0.300		
J	12'-0"	-	10	0.40##	-	12	0.40##	12	0.360		

TABLE 1 NOTES:

##Provide 0.20 eq. in.2/ft. at each face, 12" max. bar spacing.

**Modified minimum wall thickness.

***Min. total circumferential reinforcement for continuous steel hoops:

 $A_2 = 0.50$ sq. in. for riser section height equal or less than 2'-0" (2 hoop min.)

 $A_2 = 0.75$ sq. in. for riser section height more than 2'-0" up to 4'-0" (3 hoop min.) Areas of reinforcing for precast items are based on Grade 60 reinforcing;

No reduction in the area of reinforcement is allowed for welded wire fabric in Table 1; Area of vertical reinforcing may be reduced in accordance with ASTM C478.

SQUARE & RECTANGULAR STRUCTURES (ALTERNATE B) – TABLE 2							
Туре	Wall Length	Max.	Wall Thickness (t₃)				
	(ft)	Depth (ft)	CIP (in.)	Precast (in.)			
Р	≤ 3'-6"	40	6 Riser 8 Bottom	6			
J	4'-0"	40	8	6			
J	5'-0"	22	-	6			
J	6'-0"	15	-	6			
J	5'-0" to 9'-0"	40	8	8			
J	10'-0"	26	8	8			
J	10'-0" to 12'-0"	40	10	9			
J	16'-0"	35	-	9			
J	16'-0"	40	10	10			
J	20'-0"	25	-	9			
J	20'-0"	30	10	10			

TABLE 2 NOTES:

See Table 8 for Reinforcing Schedule.

GENERAL NOTES

- 1. Standard structure bottoms 4'-0" diameter and smaller (Alt. A) and 3'-6" square (Alt. B) are designated Type P. Larger standard structure bottoms are designated Type J. Risers are permitted for all structures. Round risers are designated Type A, square risers are designated Type B.
- 2. Walls of circular structures (Alt. A) constructed in place may be of brick or reinforced concrete. Precast and rectangular structures (Alt. B) shall be constructed of reinforced concrete only.
- 3. Wall thickness and reinforcement are for either reinforced cast-in-place or precast concrete units except that precast circular units may be furnished with walls in accordance with ASTM C478 (see modified wall thicknesses in Table 1).
- 4. Top and bottom slab thickness and reinforcement are for precast and cast-in-place construction. All concrete shall be of Class II concrete, except use Class IV concrete when shown in the Plans, for special applications of structures located in extremely aggressive environments. Concrete as specified in ASTM C478 (4000 psi) may be used in lieu of Class II concrete for precast items manufactured in accordance with Specifications Section 449.
- 5. All reinforcement shown is ASTM A615/A615M Grade 60 steel, deformed bar. Equivalent area Grade 40 steel or equivalent area ASTM A185 (smooth) or ASTM A497 (deformed) welded wire fabric may be substituted according to Index No. 201, unless otherwise noted.
- 6. Alt. A or Alt. B structure bottoms may be used in conjunction with curb inlet tops Types 1, 2, 3, 4, 5, 6, 9, and 10, and any manhole or junction box unless otherwise shown in the plans or other standard drawings. Alt. B structure bottoms may be used in conjunction with curb inlet Types 7 & 8, or any ditch bottom inlet unless otherwise shown in the plans or other standard drawings.
- 7. Rectangular structures may be rotated as directed by the Engineer in order to facilitate connections between the structure walls and storm sewer pipes.
- 8. Except when ACI hooks are specifically required, reinforcement in top and bottom slab shall be straight embedment.
- 9. All reinforcement must have 2" minimum cover except for 3'-6" diameter precast circular units manufactured under ASTM C478, keved construction otherwise shown. Additional bars used to restrain hole formers for precast structures with grouted pipe connections, may be left flush with the hole surface. Cut or bend reinforcement at pipe openings to maintain cover. Exposed ends of reinforcing at precast pipe openings and grouted joints must be removed to 1" below the concrete surface and sealed with a Type F epoxy in accordance with Specification Section 926. Horizontal steel in rectangular structures shall be lapped a minimum of 30 bar diameters or by standard hooks at corners.
- 10. The corner fillets shown are necessary for rectangular structures used with circular risers and inlet throats and when used on skew with rectangular risers, inlets and inlet throats. Fillets will be required in the top slab of the Alt. A structure bottoms when used with the Alt. B risers. Each fillet shall be reinforced with two #5 bars.
- 11. Inlet walls, throats, risers or manhole tops shall be secured to structures as shown on Index No. 201 (Sheet 3 of 5) Optional Construction Joints.
- 12. Structures with depths over 14' below the mean high water table are to be checked for flotation by the designer of the
- 13. Units larger than specified standards may be substituted at the contractor's option when these units will not cause or increase the severity of utility conflicts. Such larger units shall be furnished at no additional cost to the Department. Larger Alt. A units cannot replace Alt. B units without approval of the Engineer. This note applies to this Index only.
- 14. For manhole and junction box tops, for frames and covers, and, for supplementary details and notes see Index No. 201.
- 15. Type J structure bottoms must have a minimum 6'-0" wall height when possible, for maintenance access.

01/01/09

NO.

	RECTANG	ULAR	ROUND		
PIPE	Side Dimens	sion (L)	Diamet	er (D)	
SIZE	Single Pipe	Note	Single Pipe	2 to 4	
JIZL	Per Side	Number	or	Pipes	
	Per Side	Number	θ=180°	θ=90°	
18"	3'-6"		3'-6"	4'-0"	
24"	3'-6"		3'-6"	5'-0"	
30"	3'-6"/4'-0"	2	4'-0"	6'-0"	
36"	4'-0"/5'-0"	3	5'-0"	7'-0"	
42"	5'-0"		6'-0"	7'-0"	
48"	6'-0"		6'-0"	8'-0"	
54"	6'-0"		7'-0"	10'-0"	
60"	7'-0"		7'-0"	10'-0"	
66"	7'-0"/8'-0"	4	8'-0"	12'-0"	
72"	8'-0"		8'-0"	12'-0"	
78"	9'-0"		10'-0"	12'-0"	
84"	9'-0"		12'-0"	N/A	

TABLE 3 NOTES:

- 1. For Round Structures sizes with variable angles between pipes and variable pipe sizes, refer to the FDOT Storm Drain Handbook.
- 2. For 3'-6" Precast Square Structure Bottoms, 30" Pipes with similar invert elevations are not permitted in adjacent walls. Use 4'-0" Side Dimensions when 30" pipe openings are required on adjacent walls and the difference in flow lines is less than 3'-0".
- 3. For 4'-0" Precast Square Structure Bottoms, 36" Pipes with similar invert elevations are not permitted in adjacent walls. Use 5'-0" Side Dimensions when 36" pipe openings are required on adjacent walls and the difference in flow lines is less than 3'-0".
- 4. For 7'-0" Precast Square Structure Bottoms, 66" Pipes with similar invert elevations are not permitted in adjacent walls. Use 8'-0" Side Dimensions when 66" pipe openings are required on adjacent walls and the difference in flow lines is less than 4'-0".

TABL	E 4-MINIM	UM SIZES	FOR MULT	TIPLE				
PA	PARALLEL PIPE CONNECTIONS FOR							
REC	TANGULAR	R STRUCTU	JRE BOTTO	OMS				
PIPE	PIPE	MINIMUM	WALL LENGTH	H (L) FOR				
SIZE	SPACING	NUMBE	R OF PARALLEL	L PIPES				
SIZE	(5)	2	3	4				
18"	2'-10"	6'-0"	8'-6"	11'-0"				
24"	3'-5"	6'-6"	10'-0"	13'-6"				
30"	4'-3"	8'-0"	12'-6"	16'-6"				
36"	5'-1'	9'-6"	14'-6"	19'-6"				
42"	6'-0"	11'-0"	17'-0"	-				
48"	6'-9"	12'-6"	19'-0"	-				
54"	7'-8"	14'-0"	-	-				
60"	8'-6"	15'-0"	-	-				
66"	9'-0"	16'-6"	-	-				
72"	10'-0"	18'-0"	-	-				
78"	10'-9"	19'-0"	-	-				
84"	11'-8"	20'-6"	-	-				

TABLE 4 NOTES:

DESCRIPTION:

- 1. Minimum wall lengths based on precast structures, using concrete pipe with maximum skew angles per Table 5.
- 2. Wall lengths exceeding 20'-0" require special designs.

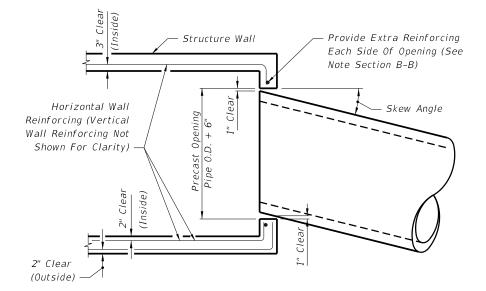
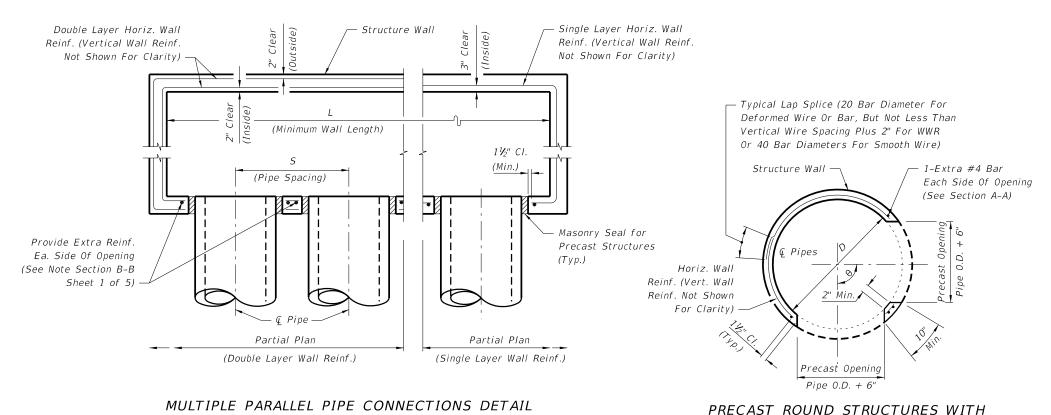


TABLE 5 - MAXIMUM PIPE SKEW FOR PRECAST ROUND OPENINGS													
	WALL	PIPE SIZE											
	THICKNESS	18"	24"	30"	36"	42"	48"	54"	60"	66"	72"	78"	84"
MAXIMUM	8"	19°	17°	16°	16°	15°	14°	14°	13°	13°	13°	12°	12°
SKEW ANGLE	6"	21°	20°	18°	17°	17°	16°	15°	15°	14°	14°	13°	13°

TABLE 5 NOTES:

These values are based on 2" clearance for precast structures. Larger skews are possible for Cast-In-Place Structures or elliptical pipe openings when approved by the Engineer.

MAXIMUM PIPE SKEW FOR PRECAST ROUND OPENINGS PLAN VIEW



STRUCTURE SIZES FOR PIPE CONNECTIONS

PLAN VIEW

REVISION 07/01/07

FDOT DESIGN STANDARDS 2013

STRUCTURE BOTTOMS TYPE J AND P

SHEET INDEXNO. NO. 3

MULTIPLE PIPE CONNECTIONS

07/01/09

$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	SHORT	Γ-WAY	LONG	G-WAY
DEPTH (Bars A) DEPTH (Bars B) SIZE: 3'-6" x UNLIMITED ≥0.5' < 24'	SLAB	SCHEDUI F	SLAB	SCHEDULF
8' < 13' 85.5 $24' + 40'$ 85.5 $13' < 31'$ $C6.5$ $31' + 40'$ $D7$ $SIZE: 4' × UNLIMITED$ $0.5' < 7'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.5'$ $0.$				
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8' < 13'	>0.5' < 8'			B10
31' < 31' C6.5 D7 B10 C6.5 C6.				
31'-40' $D7$ $B5.5$ $≥0.5' < 15'$ $B10$ $7' < 19'$ $C6.5$ $15' < 29'$ $B5.5$ $19' < 31'$ $D7$ $29'-40'$ $C6.5$ $31'-40'$ $E5$ $20.5' < 3'$ $C6.5$ $31'-40'$ $E5$ $20.5' < 3'$ $C6.5$ $3' < 7'$ $29' < 3'$ $29'-40'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20'$ $20' < 20' < 20' 20' < 20' 20' < 20' 20' < 20' < 20' 20' < 20' 20' < 20' 20' < 20' < 20' 20' < 20' 20' < 20' < 20' 20' < 20' < 20' 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20' < 20' < 20' < 20' 20$			24-40	05.5
$SIZE: 4' \times UNLIMITED$ ≥0.5' < 7'				
	31 .0	υ,		
7' < 19' C6.5 15' < 29' B5.5 19' < 31' D7 29'-40' C6.5 31'-40' E5		SIZE: 4' x	UNLIMITED	
	≥0.5′ < 7′	B5.5	≥0.5′ < 15′	B10
$SIZE: 5' \times 5'$ ≥0.5' < 3'	7' < 19'	C6.5	15' < 29'	B5.5
$SIZE: 5' \times 5'$ $\geq 0.5' < 3' $	19' < 31'	D7	29'-40'	C6.5
	31'-40'	E5		
		6175	51 51	
3' < 7' $B5.5$ $3' < 13'$ $C6.5$ $7' < 22'$ $C6.5$ $13' < 22'$ $D7$ $22' < 29'$ $D4.5$ $29' - 40'$ $E5$ $SIZE: 5' × 6'$ $≥0.5' < 12'$ $C6.5$ $≥0.5' < 3'$ $C6.5$ $12' < 26'$ $D7$ $3' < 9'$ $B5.5$ $23' < 35'$ $D4.5$ $35' + 40'$ $E5$ $23' < 35'$ $24' < 26'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ $27'$ 27				
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$SIZE: 5' \times 6'$ $\geq 0.5' < 12' $				
	29'-40'			E 5
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	26'-40'	E5		
$SIZE: 5' \times 7'$ $\geq 0.5' < 10' $				
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20' < 34'				
$SIZE: 5' \times 8'$ ≥0.5' < 7'				
$SIZE: 5' \times 8'$ $\geq 0.5' < 7' \qquad C6.5 \qquad \geq 0.5' < 8' \qquad B10$ $7' < 13' \qquad D7 \qquad 8' < 17' \qquad B5.5$ $13' < 24' \qquad E5 \qquad 17' < 25' \qquad C6.5$ $24'-40' \qquad F5 \qquad 25'-40' \qquad C3.5$ $SIZE: 5' \times 9'$ $\geq 0.5' < 8' \qquad C6.5 \qquad \geq 0.5' < 14' \qquad B10$ $8' < 14' \qquad D7 \qquad 14' < 24' \qquad B5.5$ $14' < 25' \qquad E5 \qquad 24' < 34' \qquad C6.5$ $SIZE: 5' \times UNLIMITED$ $\geq 0.5' < 8' \qquad C6.5 \qquad \geq 0.5' < 14' \qquad B10$ $8' < 14' \qquad D7 \qquad 14' < 24' \qquad B5.5$ $14' < 25' \qquad E5 \qquad 24' < 34' \qquad C6.5$ $8' < 14' \qquad D7 \qquad 14' < 24' \qquad B5.5$ $14' < 25' \qquad E5 \qquad 20.5' < 14' \qquad B10$ $14' < 25' \qquad E5 \qquad 24' < 34' \qquad C6.5$				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		SIZE:	5' x 8'	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	≥0.5′ < 7′	C6.5	≥0.5′ < 8′	B10
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	7' < 13'		8' < 17'	B5.5
$SIZE: 5' \times 9'$ $\geq 0.5' < 8' $	13' < 24'	E5	17' < 25'	C6.5
	24'-40'	F5	25'-40'	C3.5
		0.75		
$8'$ $14'$ $D7$ $14'$ $24'$ $B5.5$ $14'$ $25'$ $E5$ $24'$ $34'$ $C6.5$ $25'$ - $40'$ $F5$ $34'$ - $40'$ $C3.5$ SIZE: $5'$ x UNLIMITED $\geq 0.5'$ $8'$ $C6.5$ $\geq 0.5'$ $\geq 14'$ \geq				
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$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$SIZE: 5' \times UNLIMITED$ ≥0.5' < 8'				
≥0.5' < 8'	25'-40'	F5	34'-40'	C3.5
8' < 14' D7 14' < 24' B5.5 14' < 25' E5 24' < 34' C6.5		SIZE: 5' x	UNLIMITED	
8' < 14'	≥0.5′ < 8′	C6.5	≥0.5′ < 14′	B10
			14' < 24'	B5.5
25'-40' F5 34'-40' C3.5	14' < 25'	E5		
	25'-40'	F5	34'-40'	C3.5

SHORT	Γ-WAY	LONG	G-W AY
	I		
SLAB DEPTH	SCHEDULE (Bars A)	SLAB DEPTH	SCHEDULE (Bars B)
	SIZE:	6' x 6'	
≥0.5′ < 13′	C6.5	≥0.5′ < 10′	C3.5
13' < 23'	D7	10' < 18'	D4.5
23'-40'	E5	18' < 27'	E5
		27' < 33'	E3
		33'-40'	F5
0.51 . 01		6' x 7'	66.5
≥0.5′ < 8′	C6.5	≥0.5' < 8'	C6.5
8' < 16'	D7	8' < 12'	C3.5
16' < 28'	E5	12' < 21'	D4.5
28'-40'	F5	21' < 28'	E5
		28' < 35'	E3
		35'-40'	F5
	SIZE:	6' x 8'	
≥0.5′ < 6′	C6.5	≥0.5′ < 6′	B5.5
6' < 13'	D7	6' < 11'	C6.5
13' < 22'	E5	11' < 17'	C3.5
22' < 35'	F5	17' < 22'	D4.5
35'-40'	G5	22' < 32'	E5
		32'-40'	E3
	SIZE:	6' x 9'	
≥0.5' < 8'	D7	≥0.5′ < 8′	B5.5
8' < 14'	E5	8' < 14'	C6.5
14' < 24'	F 5	14' < 21'	C3.5
24'-34'	G5	21' < 25'	D4.5
2,3,	03	25'-34'	E5
		23 31	
	SIZE: 6' x	UNLIMITED	
≥0.5′ < 8′	D7	≥0.5′ < 8′	B5.5
8' < 14'	E5	8' < 14'	C6.5
14' < 24'	F 5	14' < 21'	C3.5
24'-34'	G5	21' < 25'	D4.5
		25'-34'	E5
		7' x 7'	
≥0.5′ < 8′	C6.5	≥0.5′ < 4′	C6.5
8' < 15'	D7	4' < 7'	C3.5
15' < 26'	E5	7' < 11'	D4.5
26'-40'	F 5	11' < 22'	E3
		22' < 32'	F3.5
		32'-40'	G3.5
	SIZE:	7' x 8'	
≥0.5′ < 5′	C6.5	≥0.5′ < 5′	C6.5
5' < 11'	D7	5' < 8'	C3.5
11' < 19'	E5	8' < 13'	D4.5
19' < 30'	F5	13' < 22'	E3
30'-40'	G5	22' < 30'	F3.5
		30'-40'	G3.5
	CIZE	7' x 9'	
	312E.		
≥0.5' < 9'	D7	≥0.5′ < 7′	C6.5
9' < 15'		7' < 10'	C3.5
9' < 15' 15' < 25'	D7 E5 F5	7' < 10' 10' < 14'	
9' < 15'	D7 E5	7' < 10' 10' < 14' 14' < 21'	C3.5
9' < 15' 15' < 25'	D7 E5 F5	7' < 10' 10' < 14'	C3.5 D4.5

SHORT	T-WAY	LONG	G-WAY
SLAB DEPTH	SCHEDULE (Bars A)	SLAB DEPTH	SCHEDULE (Bars B)
	SIZE:	8' x 8'	
≥0.5′ < 10′	D7	≥0.5' < 9'	D4.5
10' < 19'	E5	9' < 13'	E5
19'-30'	F 5	13' < 18'	F5
		18' < 23'	F3.5
		23'-30'	G3.5
	SIZE:	8' x 9'	
≥0.5′ < 8′	D7	≥0.5′ < 7′	D7
8' < 14'	E5	7' < 9'	D4.5
14' < 23'	F5	9' < 15'	E3
23'-31'	G3.5	15' < 20'	F5
		20' < 23'	F3.5
		23'-31'	G3.5
	SIZE:	9' x 9'	
≥0.5′ < 8′	D7	≥0.5′ < 7′	D4
8' < 14'	E5	7' < 10'	E5
14' < 22'	F5	10' < 17'	F3.5
		17' < 22'	G3.5
SIZ	ZE: 9'x9'x10"	SLAB THICKN	IESS
22' < 36'	F5	22' < 31'	F3.5
36'-40'	G5	31'-40'	G3.5
SIZ	E: 10'x10'x10"	SLAB THICK	NESS
≥0.5′ < 7′	C6.5	0.5' < 6'	C6.5
7' < 10'	D7	6' < 9'	D4.5
10' < 18'	E5	9' < 15'	E5
18' < 27'	F5	15' < 22'	F5
27'-32'	G5	22'-32'	G3.5
SIZ	E: 12'x12'x12"	SLAB THICK	NESS
≥0.5′ < 10′	D7	≥0.5′ < 8′	D7
10' < 16'	E5	8' < 14'	E5
16' < 25'	F5	14' < 22'	F5
25'-35'	G5	22' < 30'	G5
		30'-35'	H4

SLAB AND WALL DESIGN TABLE NOTES

- 1. Size is the inside dimension(s) of a structure.
- 2. Slab reinforcement is appropriate for top, intermediate, and bottom slabs.
- 3. Bottom Slabs for precast 3'-6" x 3'-6" rectangular structures at 15' depth or less, may be 6" thick.
- 4. Slab depth is measured from finished grade to top of slab.
- 5. Wall depth is measured to the top of the bottom slab for boxes and to the top of the intermediate slab for risers.
- lengths exceeding 5', or 10' for wall lengths exceeding 12'.

SLAB DESIGNS - ROUND STRUCTURES (TABLE 7)

SLAB DEPTH	SLAB THICKNESS	REINF. (2-WAY) SCHEDULE
SIZI	E: 3'-6" DIAMET	TER
2'-15'	6" Precast	C6.5
0.5' < 30'	8"	A6
30'-40'	8"	B5.5
	E: 4'-0" DIAMET	
≥0.5' < 19'	8"	A6
19' < 30'	8"	B5.5
30'-40'	8"	C6.5
	E: 5'-0" DIAMET	
≥0.5' < 15'	8"	B5.5
15' < 26'	8"	C6.5
26' < 35'	8"	D7
35'-40'	8"	D4.5
	E: 6'-0" DIAMET	
≥0.5′ < 9′	8"	B5.5
9' < 15'	8"	C6.5
15' < 22'	8"	C3.5
22' < 30'	8"	D4.5
30'-40'	8"	E5
	E: 7'-0" DIAMET	ER
≥0.5′ < 8′	8"	C3.5
8' < 16'	8"	D4.5
16' < 23'	8"	E5
23' < 27'	8"	E3
27'-40'	8"	F3.5
SIZI	E: 8'-0" DIAMET	TER
≥0.5′ < 10′	8"	D4.5
10' < 16'	8"	E5
16' < 19'	8"	E3
19' < 29'	8"	F3.5
29'-40'	10"	F5
SIZE	: 10'-0" DIAME	TER
≥0.5′ < 12′	10"	D4.5
12' < 20'	10"	E5
20' < 28'	10"	F5
28'-40'	10"	G3.5
SIZE	: 12'-0" DIAME	TER
≥0.5′ < 8′	10"	D4.5
8' < 13'	10"	E5
13' < 18'	10"	F5
18' < 26'	10"	G3.5
26'-40'	12"	G3.5

- 7. Wall lengths exceeding 6'-0" require two layers of reinforcing (See Table 8) with 2" of cover from the horizontal bars to the inside and outside faces for each layer.
- 8. Wall lengths exceeding the dimensions or depths shown in Table 8, or 12'-0" diameter require a special design.
- 9. Wall thickness and reinforcing for rectangular structures is the same for both long and short sides.
- 10. Reinforcing schedules with larger areas of steel may be substituted for schedules with smaller bar or wire spacing, except that Schedule B10 may not be substituted for Schedule A6. See Index 201, Sheet 4 for allowable bar spacing

6.	Wall heigh	nt is th	e dist	tance between top of lower slab to	
	bottom of	upper	slab.	Maximum wall height is 12' for wall	

adjustments when larger areas of reinforcing are substituted. INDEX HORIZONTAL

REINFORCING

SCHEDULE

WALL

VERTICAL

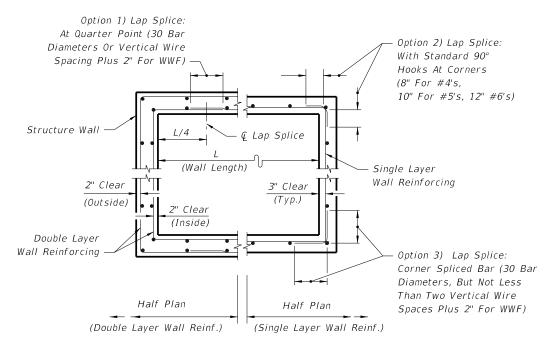
REINFORCING

SCHEDULE

VERTICAL REINFORCING			HORIZONTAL REINFORCING			WALL CKNESS
WALL DEPTH	SCHEDULE		WALL DEPTH	SCHI	EDULE	THI
SIZE: 10'-0" (Precast Only)						
	Inside	Outside		Inside	Outside	
26' - 40'	D7	D7	26' - 40'	F5	F5	9"
		SI	ZE: 12'-0"			
	Inside	Outside		Inside	Outside	
$\geq 1.17' < 14'$	B10	B10	≥1.17′ < 10′	C6.5	C6.5	10"
14' < 25'	C6.5	C6.5	10' < 17'	D7	D7	10"
25' - 40'	D7	D7	17' < 24'	E5	E5	10"
			24' - 40'	F 5	F5	10"
	SIZ	ZE: 12'-	-0" (Precast	Only)		
		Outside		Inside	Outside	
$\geq 1.17' < 12'$	B10	B10	≥1.17′ < 10′	D7	D7	9"
12' < 24'	C6.5	C6.5	10' < 17'	D4.5	D4.5	9"
24' - 40'	D7	D7	17' < 23'	E5	E5	9"
			23' < 32'	F5	F5	9"
			32' - 40'	G5	G5	9"
		SI.	ZE: 16'-0"			
		Outside		Inside	Outside	
≥1.17′ < 11′	C6.5	C6.5	≥1.17′ < 13′	D7	D7	10"
11' < 20'	D7	D7	13' < 20'	E5	E5	10"
20' < 28'	E5	E5	20' < 28'	F5	F5	10"
28' - 40'	F5	F5	28' - 40'	G5	G5	10"
	SIZ	ZE: 16'-	-0" (Precast	Only)		
	Inside	Outside		Inside	Outside	
≥1.17′ < 10′	C6.5	C6.5	≥1.17' < 9'	D7	D7	9"
10' < 18'	D7	D7	9' < 13'	D4.5	D4.5	9"
18' < 25'	E5	E5	13' < 19'	E5	E5	9"
25' - 35'	F 5	F5	19' < 27'	F5	F5	9"
			27' - 35'	G5	G5	9"
		S1.	ZE: 20'-0"			
	Inside	Outside		Inside	Outside	
≥1.17' < 10'	C6.5	C6.5	≥1.17' < 8'	D7	D7	10"
10' < 17'	D7	D7	8' < 12'	E5	E5	10"
17' - 30'	E5	E5	12' < 20'	F5	F5	10"
			20' - 30'	G5	G5	10"
	SIZ	ZE: 20'-	-0" (Precast			
		Outside			Outside	
≥1.17′ < 8′	C6.5	C6.5	≥1.17' < 8'	D4.5	D4.5	9"
8' < 13'	D7	D7	8' < 12'	E5	E5	9"
13' - 25'	E5	E5	12' < 19'	F5	F5	9"
			19' - 25'	G5	G5	9"

	REINFORCING SCHEDULE				
	GRADE 60 BARS OR 65 KSI & 70 KSI WELDED WIRE REINFORCING				
	GRADE 60	MAXIMUM SPACING			
SCHEDULE	AREA	CD 60	WWR EQUIV. AREA*		
	(in.²/ft.)	BARS (in.)	65 KSI (in.)	70 KSI (in.)	
A12	0.20	12	8	8	
A6	0.20	6	5	$4\frac{1}{2}$	
B10	0.24	10	8	7½	
B5.5	0.24	$5\frac{1}{2}$	5	4	
C6.5	0.37	$6\frac{1}{2}$	6	5	
C3.5	0.37	31/2	3	2½	
D7	0.53	7	6	5	
D4.5	0.53	$4\frac{1}{2}$	4	3½	
E5	0.73	5	4	4	
E3	0.73	3	3	3	
F 5	1.06	5	4	4	
F3.5	1.06	3½	3	3	
G5	1.45	5	4	4	
G.3.5	1.45	3½	3	3	
H4	1.75	4	3	3	

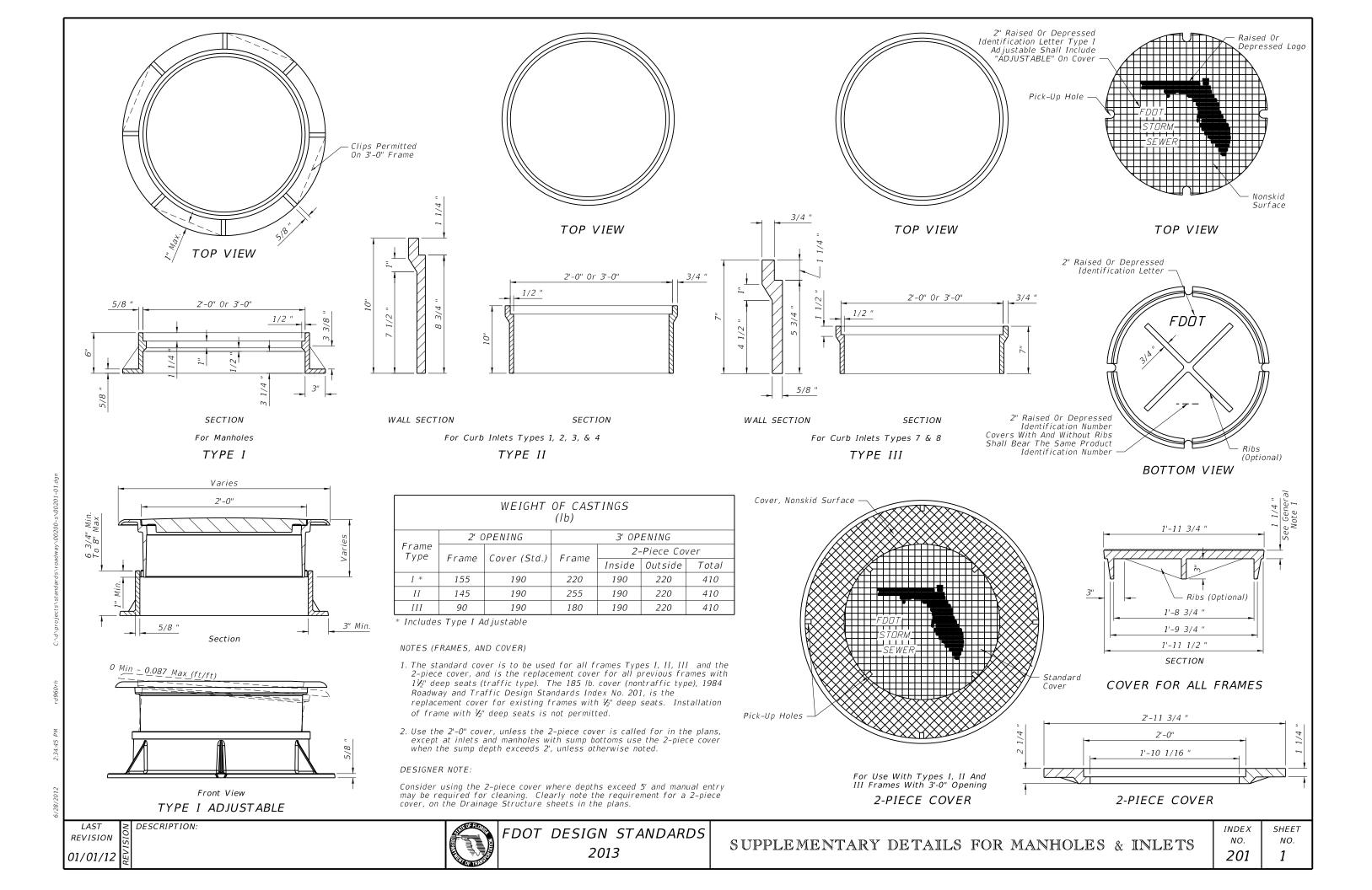
^{*}Equivalent Area Welded Wire Reinforcing may be substituted in accordance with Index No. 201, Sheet 4.



WALL REINFORCING SPLICE DETAILS (ALTERNATE B)

LAST REVISION 01/01/12



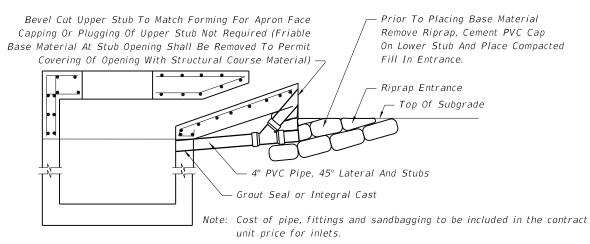


NOTE: When Alternate "G" grate is specified, the chain, bolt, nuts, washer and cold shuts shall be galvanized in accordance with Section 425 of the Standard Specifications.

Cost of eyebolt and chain to be included in the contract unit price for inlets.

EYEBOLT AND CHAIN REQUIREMENTS					
Index Number	Inlet Type	Eye- Bolts	Length Of Chain	Handling & Remarks	
	(MB) 1	1	4'-0"	Slide & Spin	
	(MB) 2	1	4'-0"	Slide & Spin	
217	(MB) 3	2	2 @ 4'-0"	Slide & Spin	
	(MB) 4	2	2 @ 4'-0"	Slide & Spin	
	(MB) 5	2	2 @ 4'-0"	Slide & Spin	
218	(BW)	1	3'-8"	Slide Or Slide & Spin	
219	(BW, RGD)	1	4'-0"	Slide & Spin	
220	5	1	4'-0"	Slide & Spin	
221	V	1	4'-0"	Slide & Spin	
230	А	1	3'-0"	Slide	
231	В	1	5'-0"	Slide & Spin	
	C	1	2'-6"	Slide & Spin	
	D	1	2'-6"	Slide & Spin	
232	Ε	2	2 @ 2'-6"	Slide & Spin	
	Н	2	2 @ 2'-6"	Flip Ctr. Grate and Slide & Spin Single Free Grate	
			1 or 2 @ 1'-6"	Ctr. Grate(s) Chained To One End Grate	
	F	1	3'-6"	Flip Or Slide & Spin	
233	G	1	6'-0"	Slide	
			2'-0"	Lifting Loop	
234	J	1	4'-0"	Slide & Spin	

EYEBOLT AND CHAIN FOR LOCKING GRATES TO INLETS



Brick Adjustment or 2' Dia. (1-Piece Cover) Brick Adjustment or Grade Ring Permitted Grade Ring Permitted 3' Dia. (2-Piece Cover, (Min. 0"-Max. 12") (Min. 0"- Max. 12") 2' Dia. (1-Piece Cover) Concrete Or 3' Dia. (2-Piece Cover) 8" Brick See Note 3 Tongue & Groove Thickness Of Joint To Match Riser $1 lac{1}{8}$ " Optional 4' Dia. Structure Wall Key Riser 4' Dia. 3'-6" Or 4' Dia.

SECTION

TYPE 7

Note: See Slab Designs Index No. 200.

BRICK OR CONCRETE PRECAST CONCENTRIC CONE PRECAST ECCENTRIC CONE TYPE 8

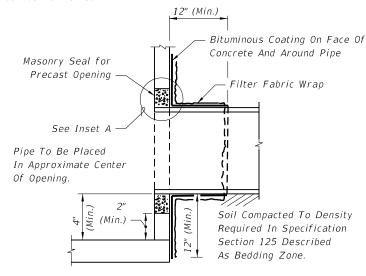
MANHOLE TOPS

NOTES (TOPS)

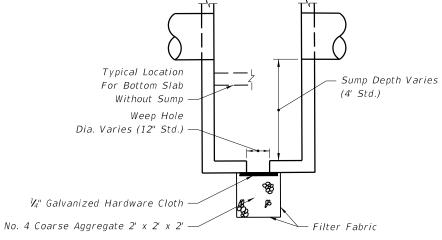
- 1. Manhole top Type 7 slabs shall be of Class II concrete. Concrete as specified in ASTM C478 may be used for precast units; see General Note No. 3.
- 2. Manhole top Type 7 slabs may be of cast-in-place or precast construction. The optional key is for precast tops and in lieu of dowels. Frame and slab openings are to be omitted when top is used over a junction box.
- 3. Manhole top Type 8 may be of cast-in-place or precast concrete construction or brick construction. For concrete construction, the concrete and steel reinforcement shall be the same as the supporting wall unit. An eccentric cone may be used.
- 4. Manhole tops shall be secured to structures by optional construction joints as shown on Sheet 3.
- 5. Frames can be adjusted a maximum 12" height with brick or precast ASTM C478 grade rings.
- 6. Substitution of manhole top Type 8 for manhole top Type 7 is allowed provided that minimum dimensions shown above are not reduced.
- 7. Substitution of Manhole top Type 7 for Type 8 is allowed if the minimum thickness (h) above pipe opening cannot be maintained with manhole top Type 8.

DESIGN NOTES

1. Manhole top Type 8 should be specified in the plans when depths shown above can be maintained.

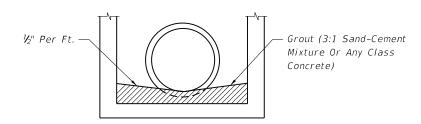


FILTER FABRIC WRAP ON GROUTED PIPE TO STRUCTURE JOINT

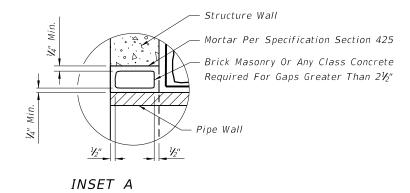


NOTE: Sump bottom appropriate for all manhole and inlet types. Sumps are to be constructed in inlet and manholes connected to French Drains unless excluded in the plans. At other locations, sump is to be constructed only where called for in the plans. Weep holes to be constructed in sump bottom only where called for in the plans. Cost of sump bottom and weep hole to be included in the contract unit price for inlet or manhole

SUMP BOTTOM



FOR ALL STRUCTURES UNLESS EXCLUDED BY SPECIAL DETAIL ALL PIPE TYPES DRAINAGE STRUCTURE INVERT



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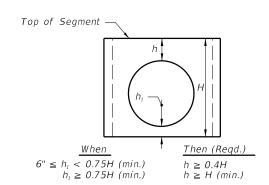
TEMPORARY DRAINS FOR SUBGRADE AND BASE

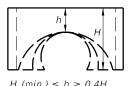
FDOT DESIGN STANDARDS 2013

SUPPLEMENTARY DETAILS FOR MANHOLES & INLETS

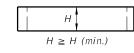
INDEX SHEET NO. NO.







Segments may be inverted. Opening for pipe shall be the pipe OD plus 6" (± 2" tolerance). If h can not be attained, then a top or bottom slab must be attached to the segment as shown below.



Minimum Value For H			
H (min.)	Box Or Riser Diameter		
1'-0"	3'-6" & 4'-0"		
1'-6"	5'-0" & 6'-0"		
2'-0"	>6'-0"		

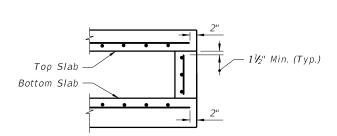
SEPARATE RISER SEGMENTS WITH CONSTRUCTION JOINTS OTHER THAN DOWEL OPTION



SEGMENTS FOR SLAB TO WALL DOWEL CONSTRUCTION JOINTS OR MONOLITHICALLY CAST SEGMENTS

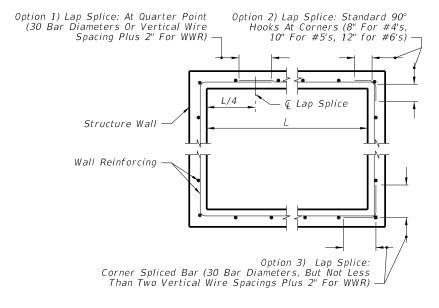
NOTE: h may be less than 6" when approved by the Engineer, but not for inlet segments at finish grade elevation

COMPARATIVE SIDE VIEWS MINIMUM DIMENSIONS FOR BOX AND RISER SEGMENTS



(NOTE: NOT APPLICABLE AROUND MANHOLE AND RISER OPENINGS)

REBAR STRAIGHT END EMBEDMENT FOR TOP AND BOTTOM SLABS

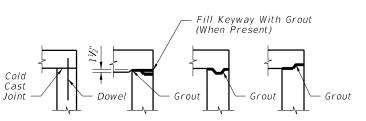


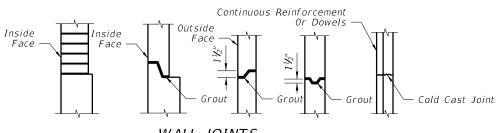
WALL REINFORCING SPLICE DETAILS

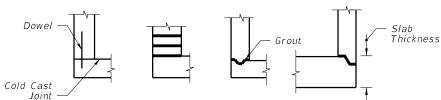
FDOT DESIGN STANDARDS 2013

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NO. 3







BOTTOM SLABS TO WALLS

- 1. One or more types of joints may be used in a single structure, except brick wall structure. Brick wall construction is permitted on circular units only.
- 2. All grouted joints are to have a maximum thickness of 1".
- 3. Keyways are to be a minimum of $1\frac{1}{2}$ " deep.

- 4. Joint dowels are to be #4 bars, 12" long with a minimum of 6 bars per joint approximately evenly spaced for circular structures or at maximum 12" spacing for rectangular structures. Bars may be either Adhesive Bonded Dowels in accordance with Specification Section 416, or placed approximately 6" into fresh concrete leaving the remainder to extend into the secondary cast. Welded wire fabric may be substituted for the dowel bar in accordance with the equivalent steel area table on
- 5. Minimum cover on dowel reinforcing bars is 2" to outside face of structure.
- 6. Joints between wall segments and between wall segments and top or bottom slabs may be sealed either by preformed plastic gasket material using the procedures given in Section 430-7.3.1 of the Specifications or by non-shrink grout, in accordance with Section 934 of the Specifications.
- 7. Approved product inserts may be used in lieu of dowel embedment.

IAST

REVISION

01/01/10

			EXAMPLE TABLE	OF EQU	IVALENT STEEL A	AREA		
	GRADE 60 REINFORCING B	AR	EQUIVALENT GRAI REINFORCING E		EQUIVALENT 65 KSI WELDED WIRE REINFO		EQUIVALENT 70 KSI DE WELDED WIRE REINFOR	
SCHEDULE	Bar Size & Spacing	Steel Area (in²/ft)	Bar Size & Spacing	Min. Steel Area (in²/ft)	Style Designation	Min. Steel Area (in²/ft)	Style Designation	Min. Steel Area (in²/ft)
А	#3 @ 6½" Ctrs. #4 @ 12" Ctrs.	0.20	#3 @ 4½" Ctrs. #4 @ 8" Ctrs. #5 @ 12" Ctrs.	0.30	3"x3"-W4.6xW4.6 4"x4"-W6.2xW6.2 6"x6"-W9.2xW9.2	0.1846	3"x3"-D4.3xD4.3 4"x4"-D5.7xD5.7 6"x6"-D8.6xD8.6	0.1714
В	#3 @ 5½" Ctrs. #4 @ 10" Ctrs.	0.24	#3 @ 3½" Ctrs. #4 @ 6½" Ctrs. #5 @ 10" Ctrs.	0.36	3"x3"-W5.5xW5.5 4"x4"-W7.4xW7.4 6"x6"-W11.1xW11.1	0.2215	3"x3"-D5.1xD5.1 4"x4"-D6.9xD6.9 6"x6"-D10.3xD10.3	0.2057
Special 1	#3 @ 5" Ctrs #4 @ 9" Ctrs.	0.267	#3 @ 3" Ctrs. #4 @ 6" Ctrs. #5 @ 9" Ctrs.	0.40	3"x3"-W6.2xW6.2 4"x4"-W8.2xW8.2 6"x6"-W12.3xW12.3	0.2465	3"x3"-D5.7xD5.7 4"x4"-D7.6xD7.6 6"x6"-D11.4xD11.4	0.2289
С	#3 @ 3½" Ctrs. #4 @ 6½" Ctrs. #5 @ 10" Ctrs.	0.37	#4 @ 4" Ctrs. #5 @ 6½" Ctrs. #6 @ 9½" Ctrs.	0.555	3"x3"-W8.5xW8.5 4"x4"-W11.4xW11.4 6"x6"-W17.1xW17.1	0.3415	3"x3"-D7.9xD7.9 4"x4"-D10.6xD10.6 6"x6"-D15.9xD15.9	0.3171
D	#4 @ 4½" Ctrs. #5 @ 7" Ctrs. #6 @ 10" Ctrs.	0.53	#4 @ 3" Ctrs. #5 @ 4½" Ctrs. #6 @ 6½" Ctrs.	0.795	3"x3"-W12.2xW12.2 4"x4"-W16.3xW16.3 6"x6"-W24.5xW24.5	0.4892	3"x3"-D11.4xD11.4 4"x4"-D15.1xD15.1 6"x6"-D22.7xD22.7	0.4543
E	#4 @ 3" Ctrs. #5 @ 5" Ctrs. #6 @ 7" Ctrs.	0.73	#5 @ 3½" Ctrs. #6 @ 4½" Ctrs. #7 @ 6½" Ctrs.	1.095	3"x3"-W16.8xW16.8 4"x4"-W22.5xW22.5 6"x6"-W33.7xW33.7	0.6738	3"x3"-D15.6xD15.6 4"x4"-D20.9xD20.9 6"x6"-D31.3xD31.3	0.6257
F	#5 @ 3½" Ctrs. #6 @ 5" Ctrs. #7 @ 7" Ctrs.	1.06	#6 @ 3" Ctrs. #7 @ 4½" Ctrs. #8 @ 6" Ctrs.	1.59	3"x3"-W24.5xW24.5 4"x4"-W32.6xW32.6 6"x6"-W48.9xW48.9	0.9785	3"x3"-D22.7xD22.7 4"x4"-D30.3xD30.3 6"x6"-D45.4xD45.4	0.9086
Special 2	#5 @ 3" Ctrs. #6 @ 4" Ctrs. #7 @ 5½" Ctrs.	1.24	#7 @ 4" Ctrs. #8 @ 5" Ctrs.	1.86	3"x3"-W28.6xW28.6 4"x4"-W38.2xW38.2 6"x6"-W57.2xW57.2	1.1446	3"x3"-D26.6xD26.6 4"x4"-D35.4xD35.4 6"x6"-D53.1xD53.1	1.0629
G	#6 @ 3½" Ctrs. #7 @ 5" Ctrs.	1.46	#7 @ 3" Ctrs. #8 @ 4" Ctrs.	2.19	3"x3"-W33.7xW33.7 4"x4"-W44.9xW44.9	1.3477	3"x3"-D31.3xD31.3 4"x4"-D41.7xD41.7	1.2514

GENERAL NOTES

- 1. For square or rectangular precast drainage structures, either deformed or smooth welded wire reinforcement may be used provided:
 - a) The smooth welded wire reinforcement shall comply with ASTM A185 and deformed welded wire reinforcement shall comply with ASTM A497.
 - b) Width and length of the unit is four times the spacing of the cross wires.
 - c) Wire reinforcement shall be continuous around the box, and lapped in accordance with Option 1 or 3 as shown in the Wall Reinforcing Splice Details.
- 2. Horizontal steel in the walls of rectangular structures shall be lap spliced in accordance with Option 1, 2 or 3 as shown in the Wall Reinforcing Splice Details.
- 3. Welding of splices and laps is permitted. The requirements and restrictions placed on welding in AASHTO M259 shall apply.
- 4. Rebar straight end embedment of peripheral reinforcement may be used in lieu of ACI standard hooks for top and bottom slabs except when hooks are specifically called for in the plans or standard drawings.
- 5. Concrete as specified in ASTM C478, (4000 psi) may be used in lieu of Class II concrete in precast items manufactured in plants which meet the requirements of Section 449 of the Specifications.
- 6. Precast opening for pipe shall be the pipe OD plus 6" (± 2" tolerance). Mortar used to seal the pipe into the opening will be of such a mix that shrinkage will not cause leakage into or out of the structure. Dry-pack mortar may be used in lieu of brick and mortar construction to seal openings less than 21/2" wide.
- 7. For pay item purposes, the height used to determine if a drainage structure is greater than 10 feet shall be computed using:
 - a) the elevation of the top of the manhole lid,
 - b) the grate elevation or the theoretical gutter grade elevation of an inlet, or
 - c) the outside top elevation of a junction box less the flow line elevation of the lowest pipe or to top of sump floor.

NOTES FOR PRECAST OPTIONS & EQUIVALENT REINFORCEMENT SUBSTITUTION

1. Details for optional precast inlet construction up to depths of 15' are shown on the inlet indexes.

- 2. When precast units are used in conjunction with Alt. "B" Structure Bottoms, Index No. 200, the interior dimensions of an Alt. "B" Bottom can be adjusted to reflect these inlet interior
- 3. Concrete which meets the requirements of ASTM C478 or Class IV must be used for precast structures constructed with 6" wall or slab thickness.
- 4. Reinforcement can be either deformed bar reinforcement or welded wire reinforcement. Bar reinforcement other than 60 ksi may be used, however only two grades are recognized; Grade 40 and Grade 60. Smooth welded wire reinforcement, will be recognized as having a design strength of 65 ksi and deformed welded wire reinforcement will be recognized as having a design strength of 70 ksi. The area of reinforcement required may be adjusted in accordance with the Equivalent Steel Area Table provided. For bars and spacings not given, the steel area required can be determined by the following equations:

Grade 40 Steel Area = $A_S40 = \frac{60}{40} \times A_S60$

#7 @ 5" Ctrs.

Smooth Welded Wire Reinforcement Steel Area = $A_565 = \underline{60} \times A_560$

Deformed Welded Wire Reinforcement Steel Area = $A_c70 = 60 \times A_c60$

When a reduced area of reinforcement is provided, any maximum bar spacing shown must also be reduced as determined by the following equations, unless otherwise shown:

Max. Grade 40 Bar Spacing = Grade 60 Bar Spacing

Max. Smooth Welded Wire Spacing = Grade 60 Bar Spacing x 0.86

Max. Deformed Welded Wire Spacing = Grade 60 Bar Spacing x 0.74

When an increased area of reinforcing is provided, then the maximum bar spacing may be increased by the squared ratio of increased steel area, but not to exceed 12":

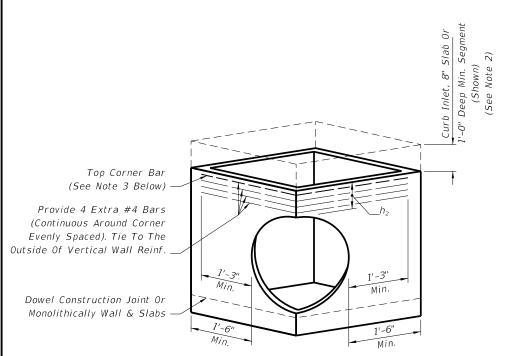
Max. Bar Spacing Provided ≤ Max. Bar Spacing Required x (Steel Area Provided Min. Steel Area Required

In no case will reinforcement with wires smaller than W3.1 or D3.1, or spacings greater than 8" be permitted. Bar reinforcement shall show the minimum yield designation grade mark or either the number 60 or one (1) grade mark line to be acceptable at the higher value. Maximum bar spacing shall not be greater than two (2) times the slab thickness with a maximum spacing of 12" or three (3) times the wall thickness, with a maximum spacing of 18" for vertical bars and 12" for horizontal bars. Wires smaller than W3.1 or D3.1 are permitted in the walls of ASTM C 478 round structure bottoms and round risers.

SHEET

NO.





 $h_2 \ge 1'-0''$ (See Notes 2 and 3 Below)

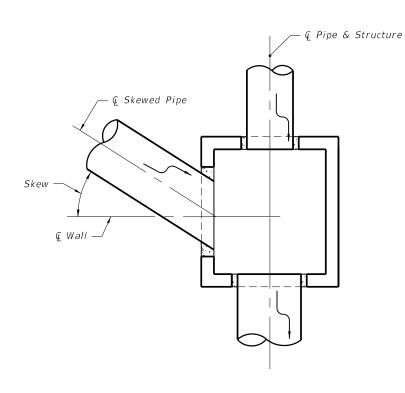
DESIGNER NOTE: Use only when round structures are not practical, engineer of record approval required.

PICTORIAL VIEW

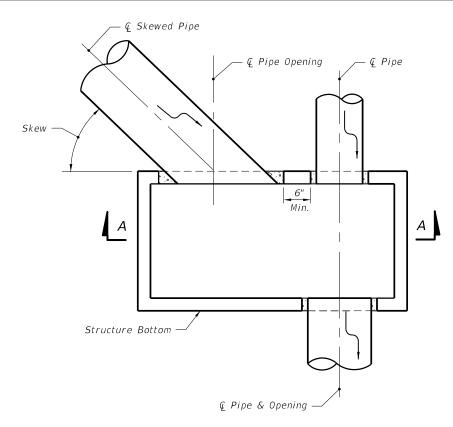
NOTE: 1. Submit Shop Drawings of corner openings for approval by the Engineer of Record.

- 2. h_2 may be less than 1'-0" when a minimum 1'-0" deep segment, 8" slab or curb inlet is provided above the corner opening.
- 3. For inlet segments at finish grade elevation substitute a #8 Bar for the top corner bar when $1'-0'' \le h_2 < 2'-0''$.

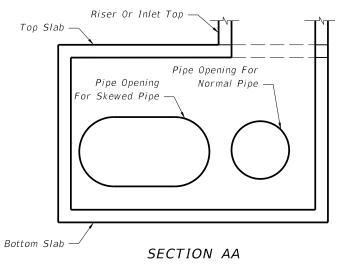
RECTANGULAR SEGMENT WITH PIPE OPENING AT CORNER



PLAN VIEW FOR SKEWS ≤ 45° (Not Centered)



PLAN VIEW FOR SKEWS > 45° (Not Centered)



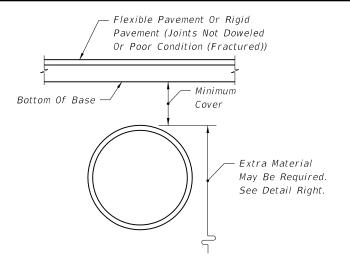
(Pipes Not Shown For Clarity)

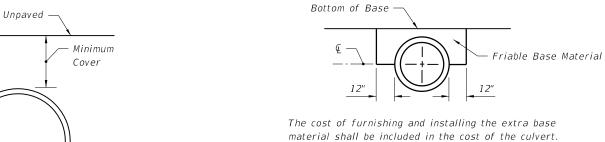
DETAILS FOR SKEWED PIPES IN RECTANGULAR STRUCTURES



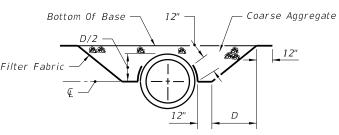
INDEX

SHEET





FRIABLE BASE



The coarse aggregate shall be placed in 6 inch lifts and compacted sufficiently as to be firm and unyielding. The coarse aggregate shall be gravel or stone meeting the requirements of Standard Specification Sections 901-2 or 901-3 respectively. The gradation shall meet Section 901-1.4, Grades 4, 467, 5, 56, or 57 unless restricted in the plans. The filter fabric shall be Type D-3 (See Index No. 199). The cost of furnishing and installing the coarse aggregate and filter fabric shall be included in the cost of the culvert.

ASPHALTIC CONCRETE BASE

Note: Extra material is required when cross culverts are located on facilities subject to high speed traffic (≥ 55 mph) or high traffic volumes (> 1600 ADT) and the cover is less than 12 inches for concrete pipe, 15 inches for corrugated steel pipe, and 18 inchesfor corrugated aluminum pipe, corrugated polyethylene and corrugated polyvinyl chloride pipe.

EXTRA MATERIAL FOR CROSS CULVERTS UNDER FLEXIBLE PAVEMENTS

GENERAL NOTES

- 1. The tabulated values are recommended minimum dimensions to withstand anticipated highway traffic loads. Additional cover may be required to support construction equipment loads or highway traffic loads before pavement is completed. Some size thickness combinations may require minimum cover greater than those listed above. See Sheets 2, 3, & 4.
- 2. Less than the tabulated minimum cover may be used provided suitable method(s) are detailed in the plans.
- 3. Values shown in parenthesis () are for 3" x 1" corrugations which must be specified to utilize the lesser cover.
- 4. The tabulated values in the brackets [] apply to Type 1-R (Spiral Rib) pipe which must be specified to utilize the lesser cover.
- 5. Commercial and noncommercial refers to typical vehicular utilization of unpaved roads and drives where rutting and cover displacement may occur.
- 6. For Pipe Class S with diameters of 12" to 30", the minimum height of fill measured from top of finished grade to outside top of pipe is 3 feet.

RIGID PAVEMENT

PIPE TYPE/SIZE & SHAPE	MINIMUM COVER
CONCRETE (See Note 6)	
Round & Elliptical	9"
CORRUGATED STEEL	
15"–72" Round & Arch Equiv.	9"
78" & Larger Round & Arch Eq.	15"
CORRUGATED ALUMINUM	
15"–72" Round & Arch Equiv.	9"
78"-102" Round & Arch Equiv.	15"
108" & Larger Round	18"
CORRUGATED POLYETHYLENE	
15"–60" Round	9"
POLYVINYL CHLORIDE	
15"–36" Round	9"

FLEXIBLE	PAVEMENT

PIPE TYPE/SIZE & SHAPE	MINIMUM COVER
CONCRETE (See Note 6)	
Round & Elliptical	7"
CORRUGATED STEEL	
12"-30" Round	12" [12"]
36"-48" Round	18" (12") [15"]
54"-72" Round	21" (15") [18"]
78"-96" Round	(18") [27"]
102" & Larger Round	(24") [33"]
15"–30" Arch Equiv.	18" [18"]
36"–48" Arch Equiv.	24" (12") [18"]
54"–72" Arch Equiv.	27" (15") [24"]
78"-96" Arch Equiv.	(18") [30"]
102" & Larger Arch Equiv.	(24")
CORRUGATED ALUMINUM	
12"-24" Round	15" [12"]
30"-48" Round	18" (12") [18"]
54"-72" Round	24" (18") [24"]
78"–102" Round	(24") [30"]
108" & Larger	(30")
15"–24" Arch Equiv.	24" [21"]
30"–48" Arch Equiv.	27" (15") [24"]
54"–72" Arch Equiv.	30" (18") [27"]
78"–90" Arch Equiv.	(24") [30"]
96"-102" Arch Equiv.	(30")
CORRUGATED POLYETHYLENE	
15"-60" Round	15"
POLYVINYL CHLORIDE	
15"-36" Round	15"

	MINIMUN	1 COVER
PIPE TYPE/SIZE & SHAPE	COMMERCIAL	NON- COMMERCIAL
CONCRETE (See Note 6)		
Round & Elliptical	12"	3"
CORRUGATED STEEL		
12"–30" Round	18" [15"]	12" [12"]
36"-48" Round	18" (12") [15"]	12" (12") [12"]
54"-72" Round	18" (12") [15"]	15" (12") [12"]
78"-96" Round	(18") [27"]	(12") [12"]
102" & Larger Round	24" [33"]	18" [21"]
15"–30" Arch Equiv.	18" [18"]	12" [12"]
36"–48" Arch Equiv.	24" (12") [21"]	18" (12") [15"]
54"–72" Arch Equiv.	30" (18") [24"]	24" (12") [18"]
78"–96" Arch Equiv.	(24") [27"]	(18") [21"]
102" & Larger Arch Equiv.	(30")	(24")
CORRUGATED ALUMINUM		
12"-24" Round	21" [21"]	15" [15"]
30"–48" Round	24" (18") [21"]	18" (12") [15"]
54"-72" Round	30" (24") [27"]	24" (18") [21"]
78"–102" Round	(30") [33"]	(24") [27"]
108" & Larger	36"	30"
15"–24" Arch Equiv.	27" [24"]	24" [21"]
30"–48" Arch Equiv.	33" (21") [27"]	27" (15") [21"]
54"-72" Arch Equiv.	36" (24") [30"]	30" (18") [24"]
78"–90" Arch Equiv.	(30") [36"]	(24") [30"]
96"-102" Arch Equiv.	(36")	(30")
CORRUGATED POLYETHYLENE		
15"–60" Round	21"	15"
POLYVINYL CHLORIDE		
15"–36" Round	21"	15"

UNPAVED

MINIMUM COVER FOR CONCRETE, STEEL, ALUMINUM, POLYETHYLENE AND POLYVINYL CHLORIDE PIPE

LAST REVISION 07/01/07

DESCRIPTION:



DOT DESIGN STANDARDS 2013

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ROUND PIPE DIMENSIONS				
Equiv. Dia. (In.)	Area (Sq. Ft.)	Wall Thickness (In.)* CLASSES II, III, IV, V B WALL		
12	0.8	2		
15	1.2	21/4		
18	1.8	21/2		
24	3.1	3		
30	4.9	3½		
36	7.1	4		
42	9.6	4⅓		
48	12.6	5		
54	15.9	5⅓		
60	19.6	6		
66	23.8	$6 lac{1}{2}$		
72	28.3	7		
78	33.2	7⅓		
84	38.5	8		
90	44.4	8⅓		
96	50.3	9		
102	56.7	9½		
108	63.7	10		
114	70.9			
120	78.5			

^{*} For Informational Purposes Only. Do Not Specify Wall Thickness. Option B Wall Is Industry Standard.

Non	ninal D	imens	ions			Wall
Но	riz.	Vε	ert.	Equiv.	Area	Thickness (In.)
Rise (In.)	Span (In.)	Rise (In.)	Span (In.)	Dia. (In.)	(Sq.Ft.)	Classes HE II, III, IV VE II, III, IV
NA	NA	NA	NA	12	NA	NA
12	18	18	12	15	1.3	21/2
14	23	23	14	18	1.8	2¾
19	30	30	19	24	3.3	31⁄ ₄
24	38	38	24	30	5.1	3¾
29	45	45	29	36	7.4	41/2
34	53	53	34	42	10.2	5
38	60	60	38	48	12.9	5½
43	68	68	43	54	16.6	6
48	76	76	48	60	20.5	6½
53	83	83	53	66	24.8	7
58	91	91	58	72	29.5	7½
63	98	98	63	78	34.6	8
68	106	106	68	84	40.1	8½
72	113	113	72	90	46.1	9
77	121	121	77	96	52.4	91/2
82	128	128	82	102	59.2	10
87	136	136	87	108	66.4	10½
92	143	143	92	114	74.0	11
97	151	151	97	120	82.0	111/2

For	Informational	Purposes	Only
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ROUND PIPE INSTALLATIONS								
PIPE		Maxim	um Heig	ght of F	ill (ft.)			
DIAMETER	Class S	Class I	Class II	Class III	Class IV	Class V		
12"-30"	9	13	17	24	36	55		
36"-54"	8	12	16	22	34	52		
60"-78"	7	11	15	21	33	51		
84"-96"	6	10	14	20	32	49		
'	D-Load= D-Load=			•				
Pipe Class I D-Load=800 Lbs./Ft./Ft. (0.01" Crack) D-Load=1200 Lbs./Ft./Ft. (Ultimate)								
Pipe Class II D-Load=1000 Lbs./Ft./Ft. (0.01" Crack) D-Load=1500 Lbs./Ft./Ft. (Ultimate)								
'	D-Load= D-Load=				,			

Note: At the option of the pipe supplier or the contractor, a Pipe Class with greater strength may be substituted for the Pipe Class designated in the plans.

Pipe Class IV D-Load=2000 Lbs./Ft./Ft. (0.01" Crack)

Pipe Class V

D-Load=3000 Lbs./Ft./Ft. (Ultimate)
D-Load=3000 Lbs./Ft./Ft. (0.01" Crack)

D-Load=3750 Lbs./Ft./Ft. (Ultimate)

ELLIPTICAL PIPE INSTALLATIONS (All Sizes)

Installation	Maximum Height Of Fill (Ft.)	Pipe Class	Bedding Class	
Horizontal	1-6*	HE II*	C	
	7-10	HE III	C	
	11-16	HE IV	C	
	17+	Special Design	Modified	
Vertical	1-6*	VE II*	C	
	7-10	VE III	C	
	11-16	VE IV	C	
	17+	Special Design	Modified	

Pipe Class HE II D-Load=1000 Lbs./Ft./Ft. (0.01" Crack)
And VE II D-Load=1500 Lbs./Ft./Ft. (Ultimate)

Pipe Class HE III D-Load=1350 Lbs./Ft./Ft. (0.01" Crack)
And VE III D-Load=2000 Lbs./Ft./Ft. (Ultimate)

Pipe Class HE IV D-Load=2000 Lbs./Ft./Ft. (0.01" Crack)
And VE IV D-Load=3000 Lbs./Ft./Ft. (Ultimate)

* Note: HE III and VE III pipe required for depths of cover less than 2' for 15", 18" and 24" equivalent.

MAXIMUM COVER HEIGHTS CONCRETE PIPE

Note: Height of fill (maximum cover) is measured from top of finished grade to outside top of pipe.

PIPE DIMENSIONS CONCRETE PIPE

POLYETHYLENE PIPE						
DIAMETER	HEIGHT OF MAXIMUM FILL (Ft.)					
12"-60"	17'					

POLYV	POLYVINYL CHLORIDE PIPE						
DIAMETER	HEIGHT OF MAXIMUM FILL (Ft.)						
12"-36"	17'						

MAXIMUM COVER FOR PLASTIC PIPE

LAST DESCRIPTION:
REVISION 07/01/09



ROUND PIPE - 3" x 1" CORRUGATION									
D (In.)	Area (Sg. Ft.)	Mā S	Min. Cover						
(111.)	(34. 11.)	0.064 (16)	0.079 (14)	(Gage) 0.109 (12)	0.138 (10)	0.168 (8)	(Ft.)		
36	7.1	81	100+	100+	NA	NA			
42	9.6	70	87	100+	NA	NA	1		
48	12.6	61	76	100+	100+	NA			
54	16.0	54	68	95	100+	NA			
60	19.6	48	61	85	100+	NA	See		
66	23.8	44	55	78	100	100+*	Sheet		
72	28.3	40	51	71	91	100+*	1 of 6		
78	33.2	37	47	66	84	100+*			
84	38.5	35	43	61	78	100+*			
90	44.2	32	40	57	73	90*			
96	50.3	NS	38	53	68	84*	1		
102	56.7	NS	36	50	64	79*]		
108	63.6	NS	NS	47	61	75*]		
114	70.9	NS	NS	45	58	71*]		
120	78.5	NS	NS	42	55	67*]		
132	95.0	NS	NS	NS	50	61*	1		

			Maximum Height Of Fill (Ft.)					
D	Area	S	heet Th	ickness	In Inche	e <i>s</i>	Min.	
(In.)	(Sq. Ft.)			(Gage)			Cove	
		0.064	0.079	0.109	0.138	0.168	(Ft.)	
26		(16)	(14)	(12)	(10)	(8)		
36	7.1	72	90	100+	NA	NA		
42	9.6	62	77	100+	NA	NA		
48	12.6	54	68	95	100+	NA		
54	16.0	48	60	84	100+	NA		
60	19.6	43	54	76	98	NA	See	
66	23.8	39	49	69	89	100+*	Shee	
72	28.3	36	45	63	81	100*	1 of	
78	33.2	33	41	58	75	92*		
84	38.5	31	38	54	70	85*		
90	44.2	29	36	50	65	80*		
96	50.3	NS	34	47	61	75*		
102	56.7	NS	32	44	57	70*		
108	63.6	NS	NS	42	54	66*		
114	70.9	NS	NS	40	51	63*		
120	78.5	NS	NS	38	49	60*		
132	95.0	NS	NS	NS	44	54*		

Notes:

Increase the minimum cover values shown on Sheet 1 of 6 by 6" for gage and size combinations below the heavy lines.

Height of fill (maximum cover) is measured from top of finished grade to outside of pipe.

- * Recorrugated end not available. May be considered for cross drain and side drain applications only. NA-Not Available NS-Not Suitable (For Highway H-20 or HS-20 Loadings)
- ① Limited availability of this product. Check availability before specifying (generally limited to 3" x 1" corrugation pipe arch fabricated from 60" and smaller diameter round pipe in 12 ga. and thicker material).
- ② 360° perforated pipe arch (french drain pipe) is not recommended. Do not specify without checking suitability and availability.
- ③ 5" x 1" corrugated pipe is currently not manufactured for the Florida market. Check availability before specifying.

PIPE ARCH: SPIRAL RIB: 3/4" x 3/4" x 71/2" RIB SPACING PIPE ARCH: SPIRAL RIB: 3/4" X1" X111/2" RIB SPACING PIPE ARCH - $2\frac{2}{3}$ " $x\frac{1}{2}$ " CORRUGATION

Span (In.)	Rise (In.)	Equiv. Round Pipe (In.)	Area (Ft.²)	Minimum Sheet Thickness Required (In.) (Ga.)	Of Fi Maximu Pre	m Height ill (Ft.) m Corner ssure ./Ft.² 6000	Min. Cover (Ft.)
17	13	15	1.1	0.064 (16)	12	14	
21	15	18	1.6	0.064 (16)	10	14	1
24	18	21	2.2	0.064 (16)	7	13	
28	20	24	2.9	0.064 (16)	5	11	See
35	24	30	4.5	0.064 (16)	NS	7	Sheet
42	29	36	6.5	0.064 (16)	NS	7	1 of 6
49	33	42	8.9	0.079 (14)	NS	6	1
57	38	48	11.6	0.109 (12)	NS	8	1
64	43	54	14.7	0.109 (12)	NS	9]
71	47	60	18.1	0.138 (10)	NS	10]
77	52	66	21.9	0.168 (8)*	5	10]
83	57	72	26.0	0.168 (8)*	5	10	

PIPE ARCH-3" x 1" 123 and 5" x 1" 23 CORR.									
Span (In.)	Rise (In.)	Equiv. Round Pipe (In.)	Area (Ft.²)	Minimum Sheet Thickness Required (In.) (Ga.)	Maximun Of Fil Maximun Pres Lbs.,	I (Ft.) n Corner sure /Ft. ²	Min. Cover (Ft.)		
40	31	36	7.0	0.079 (14)	4000 8	6000 12			
46	36	42	9.4	0.079 (14)	8	13			
53	41	48	12.3	0.079 (14)	8	13			
60	46	54	15.6	0.079 (14)	8	13			
66	51	60	19.3	0.079 (14)	9	13			
73	55	66	23.2	0.079 (14)	11	16	See		
81	59	72	27.4	0.079 (14)	11	17	Sheet		
87	63	78	32.1	0.079 (14)	10	16	1 of 6		
95	67	84	37.0	0.079 (14)	11	17			
103	71	90	42.4	0.109 (12)	10	15			
112	75	96	48.0	0.109 (12)	10	16			
117	79	102	54.2	0.109 (12)	10	15			
128	83	108	60.5	0.138 (10)	9	14			
137	87	114	67.4	0.138 (10)	8	13			
142	91	120	74.5	0.168 (8)	7	12			

ROUND PIPE - SPIRAL RIB
RIB SPACING $(\frac{3}{4}"x\frac{3}{4}"x7\frac{1}{2}")$ or $(\frac{3}{4}"x1"x11\frac{1}{2}")$

	Maximum Height Of Fill (Ft.)								
D	Area	S	heet Th	ickness	In Inche	25	Min. Cover		
(In.)	(Sq. Ft.)		(Gage)						
()	(- 4 ,	0.064	0.079	0.109	0.138	0.168	(Ft.)		
		(16)	(14)	(12)	(10)	(8)			
12	0.79	NA	NA	NA	NA	NA			
15	1.23	NA	NA	NA	NA	NA			
18	1.77	68	72	NA	NA	NA			
21	2.40	58	62	100+	NA	NA			
24	3.14	51	72	100+	100+	NA			
30	4.91	41	58	97	100+	NA			
36	7.1	34	48	81	100+	NA	See		
42	9.6	29	41	69	100+	NA	Sheet		
48	12.6	26	36	61	100+	NA	1 of 6		
54	16.0	23	32	54	91	NA			
60	19.6	NS	29	49	83	NA			
66	23.8	NS	26	44	74	NA			
72	28.3	NS	24	40	67	NA			
78	33.2	NS	NS	37	59	NA			
84	38.5	NS	NS	35	54	NA			
90	44.2	NS	NS	32	48	NA			
96	50.3	NS	NS	30	45	NA			
102	56.7	NS	NS	29	40	NA			
108	63.6	NS	NS	27 A	36	NA			

 $(A) = \frac{3}{4}$ " $\times 1$ " $\times 11\frac{1}{2}$ " Only

MAXIMUM COVER FOR CORRUGATED STEEL PIPE ROUND AND PIPE ARCH

LAST REVISION 07/01/09

DESCRIPTION:



FDOT DESIGN STANDARDS 2013

INDEX NO. 205 3

SHEET NO.

07/01/07

ı	ROUND PIPE - $2\frac{2}{3}$ " x $\frac{1}{2}$ " CORRUGATION										
D (In.)	Area	M	laximum	Height O	of Fill (Ft	·.)	Min.				
	Area	She	Sheet Thickness In Inches (Gage)								
	(Sq. Ft.)	0.060 (16)	0.075 (14)	0.105 (12)	0.135 (10)	0.164 (8)	(Ft.)				
12	0.8	90	100+	NA	NA	NA					
15	1.2	72	90	NA	NA	NA					
18	1.8	59	75	100+	NA	NA					
21	2.4	52	65	92	NA	NA					
24	3.1	44	56	79	NA	NA	See				
30	4.9	35 DR	44	63	NA	NA	Sheet				
36	7.1	NS	36 DR	52	68	NA	1 of 6				
42	9.6	NS	NS	44 DR	58	NA					
48	12.6	NS	NS	38 DR	50 DR	61					
54	15.9	NS	NS	34 DR	45 DR	54 DR]				
60	19.6	NS	NS	NS	39 DR	49 DR					
66	23.8	NS	NS	NS	NS	44 DR					
72	28.3	NS	NS	NS	NS	40 DR	1				

ROUND PIPE - 3" x 1" CORRUGATION										
		٨	1aximum	Height	Of Fill (F	t.)	Min.			
D (In.)	Area (Sq. Ft.)	She	Sheet Thickness In Inches (Gage)							
(111.)	(= 4 ,	0.060 (16)	0.075 (14)	0.105 (12)	0.135 (10)	0.164 (8)	(Ft.)			
36	7.1	33	42	60	NA	NA				
42	9.6	28	36	51	NA	NA				
48	12.6	24	31	45	58	NA				
54	15.9	21	28	39	51	NA				
60	19.6	19	24	35	46	NA	See			
66	23.8	15 DR	22	32	42	51	Sheet			
72	28.3	NS	20 DR	29	38	47	1 of 6			
78	33.2	NS	15 DR	27	35	43				
84	38.5	NS	NS	24 DR	32	40				
90	44.2	NS	NS	23 DR	30	37				
96	50.3	NS	NS	21 DR	28 DR	34				
102	56.7	NS	NS	NS	26 DR	32				
108	63.6	NS	NS	NS	24 DR	30 DR				
114	70.9	NS	NS	NS	NS	28 DR				
120	78.5	NS	NS	NS	NS	27 DR				

	ROUND PIPE - SPIRAL RIB RIB SPACING (¾" x ¾" x 1½")										
D	Area (Sq. Ft.)	Ma	aximum	Height (Of Fill (Ft	.)	Min.				
(In.)		Shee	t Thickr	ness In .	Inches (G	age)	Cover				
(111.)		0.060	0.075	0.105	0.135	0.164	(Ft.)				
		(16)	(14)	(12)	(10)	(8)					
12	0.79	NA	NA	NA	NA	NA					
15	1.23	63 (Ī)	87 ①	NA	NA	NA					
18	1.77	55	76	NA	NA	NA					
21	2.40	47	65	NA	NA	NA					
24	3.14	41	57	NA	NA	NA					
30	4.91	33 DR	45	73	NA	NA	See				
36	7.1	//27 4 //	38 DR	61	NA	NA	Sheet				
42	9.6	NS	32 4	52	NA	NA	1 of 6				
48	12.6	NS	NS	46	65	NA					
54	16.0	NS	NS	40 DR	57	NA					
60	19.6	NS	NS	36 4	52	NA					
66	23.8	NS	NS	NS	47 DR	NA					
72	28.3	NS	NS	NS	///43 4	NA					
78	33.2	NS	NS	NS	39 4	NA					
84	38.5	NS	NS	NS	34 4	NA					
90	44.2	NS	NS	NS	30(1)(3)(4)	NA					

50.3 NS NS NS 27 13 4 NA

Special installation required. Refer to AASHTO Standard Specifications for Highway Bridges or ASTM B788-88 and manufacturer's recommendations.

PIPE ARCH - $2\frac{1}{2}$ " × $\frac{1}{2}$ " CORRUGATION ②										
		Equiv.		Minimum Sheet	Maximum Of Fill	9	Min.			
Span (In.)	Rise (In.)	Round Pipe (In.)	Area Thickness (Sq. Ft.) Required (In.) (Ga.)		Maximum Pressure-Ll		Cover (Ft.)			
				(In.) (Ga.)	4000	6000				
17	13	15	1.1	0.060 (16)	12	15				
21	15	18	1.6	0.060 (16)	10	14				
24	18	21	2.2	0.060 (16)	7	13				
28	20	24	2.9	0.075 (14)	5	11				
35	24	30	4.5	0.075 (14)	NS	7]			
42	29	36	6.5	0.105 (12)	NS	7	See			
49	33	42	8.9	0.105 (12)	NS	6	Sheet 1 of 6			
57	38	48	11.6	0.135 (10)	NS	8	1 1 01 0			
64	43	54	14.7	0.135 (10)	NS	9	1			
71	47	60	18.1	0.164 (8)	NS	10]			
77	52	66	21.9	0.164 (8)	NS	10	1			
83	57	72	26.0	0.164 (8)	NS	10				

PIPE ARCH - 3" x 1" CORRUGATION ①②										
		Equiv.		Minimum Sheet	Maximum Of Fill	_	Min.			
Span (In.)	Rise (In.)	Round Pipe (In.)	Round Area Thick Pipe (Sq. Ft.) Requ		Maximum Pressure-Lk		Cover (Ft.)			
				(In.) (Ga.)	4000	6000				
40	31	36	7.0	0.060 (16)	8	12				
46	36	42	9.4	0.060 (16)	8	13				
53	41	48	12.3	0.060 (16)	8	13				
60	46	54	15.6	0.075 (14)	8	13				
66	51	60	19.3	0.075 (14)	8	13]			
73	55	66	23.2	0.105 (12)	11	16	See Sheet			
81	59	72	27.4	0.105 (12)	11	17	1 of 6			
87	63	78	32.1	0.105 (12)	10	16	1 0 0			
95	67	84	37.0	0.105 (12)	11	17	1			
103	71	90	42.4	0.135 (10)	10	15				
112	75	96	48.0	0.135 (10)	10	16				
117	79	102	54.2	0.164 (8)	10	15				

PIPE ARCH - SPIRAL RIB RIB SPACING (¾" x ¾" x 7½")										
Span (In.)	Rise (In.)	Equiv. Round Pipe (In.)	Area (Sq. Ft.)	Minimum Sheet Thickness Required	Maximum Height Of Fill (Ft.) Maximum Corner Pressure-Lbs./Sq.Ft.		Min. Cover (Ft.)			
	,,,,	(171.)		(In.) (Ga.)	4000	6000				
16	14	15	1.2	0.060 (16)	12	13				
20	16	18	1.7	0.060 (16)	10	12				
23	19	21	2.3	0.060 (16)	7	1 1				
27	21	24	3.0	0.060 (16)	5	10				
33	26	30	4.7	0.075 (14)	NS	9	l			
40	31	36	7.0	0.075 (14)	NS	8	See			
46	36	42	9.4	0.105 (12)	NS	8	Sheet 1 of 6			
53	41	48	12.3	0.105 (12)	NS	8	1 01 6			
60	46	54	15.6	0.105 (10)	NS	8				
66	51	60	19.3	0.135 (10)	NS	8				
73	55	66	23.2	0.135 (10)/4)	NS	8	1			
81	59	72	27.4	0.135 (10)/4)/	NS	8				

MAXIMUM COVER FOR CORRUGATED ALUMINUM ALLOY ROUND PIPE AND PIPE ARCH

Increase the minimum cover values shown on Sheet 1 of 6 by 6" for gage and size combinations below the heavy lines.

Height of fill (maximum cover) is measured from top of finished grade to outside top of pipe.

DESCRIPTION:

NA-Not Available

NS-Not Suitable (For Highway H-20 or HS-20 Loadings)

DR-Design Review is recommended for each specific application. The review should identify any special handling, installation, backfill procedures, and construction load restrictions which may be required. (The review performed by the designer does not relieve the contractor from analyzing and taking any necessary precautions required to protect partially or completely constructed pipe from the equipment used during construction.) (NOTE: The DESIGNER may use a thicker gage in lieu of the Design Review.)

- ① Limited availability of this product. Check availability before specifying.
- 2) 360° perforated pipe (french drain pipe) is not recommended in the pipe arch shape. Do not specify without checking both for suitability and availability.
- 3 This size and gage combination must be strutted during installation per manufacturer's recommendations. Extra care will be required during handling and installation.
- ④ Use of this size and gage combination must be approved by the State Drainage Engineer.

FDOT DESIGN STANDARDS 2013

SHEET INDEXNO. NO. 205

Aluminum Structural Plate Height of Cover Limits* Combination Metal Thickness, Reinforcing Rib Type, and Rib Spacing Arch Shape- HS 20 Live Load

					Minimum Heig	ht of Cover (Ft.)		
Span (FtIn.)	Rise (FtIn.)	Area (Sq.Ft.)	1.00	1.50	2.00	2.50	3.00	3.50
5-0	1-9 2-3 2-7	7 9 10	0.125 (45)	0.100 (31)	0.100 (31)	0.100 (31)	0.100 (31)	0.100 (31)
6-0	1-10 2-4 2-9 3-2	8 10 13 15	0.125-II-18 (37)	0.100 (25)	0.100 (25)	0.100 (25)	0.100 (25)	0.100 (25)
7-0	2-4 2-10 3-3 3-8	12 15 18 20	0.125-II-18 (32)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)
8-0	2-11 3-4 4-2	17 20 26	0.125-II-9 (28)	0.150 (37)	0.100 (19)	0.100 (19)	0.100 (19)	0.100 (19)
9-0	2-11 3-10 4-8	19 26 33	0.125-IV-9 (25)	0.125-II-18 (25)	0.100 (17)	0.100 (17)	0.100 (17)	0.100 (17)
10-0	3-6 4-5 5-2	25 33 41	0.125-IV-9 (22)	0.125-II-18 (22)	0.125 (22)	0.100 (15)	0.100 (15)	0.100 (15)
11-0	3-6 4-6 5-8	28 37 50	0.175-IV-9 (32)	0.125-II-18 (20)	0.125-II-27 (20)	0.100 (14)	0.100 (14)	0.100 (14)
12-0	4-1 5-0 6-3	35 45 59		0.125-IV-18 (18)	0.125-II-27 (18)	0.125 (18)	0.100 (12)	0.100 (12)
13-0	4-1 5-1 5-11 6-9	38 49 59 70		0.150-IV-18 (23)	0.125-II-27 (17)	0.150 (23)	0.100 (11)	0.100 (11)
14-0	4-8 5-7 6-5 7-3	47 58 70 81		0.125-IV-9 (16)	0.125-IV-27 (16)	0.125-11-27 (16)	0.100 (11)	0.100 (11)
15-0	4-8 5-8 6-7 7-5 7-9	50 63 75 87 93		0.125-IV-9 (15)	0.125-IV-27 (15)	0.125-11-27 (15)	0.125 (15)	0.125 (15)
16-0	5-3 6-2 7-1 7-11 8-3	60 73 86 99 105		0.150-IV-9 (18)	0.125-IV-18 (14)	0.125-11-27 (14)	0.150 (18)	0.125 (14)
17-0	5-3 6-3 7-2 8-0 8-10	64 78 92 105 119		0.225-IV-9 (27)	0.150-IV-18 (17)	0.125-11-27 (13)	0.175 (20)	0.150 (17)
18-0	5-9 6-9 7-8 8-6 8-11	75 90 105 119 126			0.175-IV-18 (19)	0.125-IV-27 (12)	0.200 (22)	0.175 (19)
19-0	6-4 7-4 8-2 9-0 9-5	87 103 118 133 141			0.125-IV-9 (11)	0.125-IV-27 (11)	0.125-IV-54 (11)	0.125-IV-54 (11)

Aluminum Structural Plate Height of Cover Limits* Combination Metal Thickness, Reinforcing Rib Type, and Rib Spacing Round Shape- HS 20 Live Load

				Minimum Height of Cover (Ft.)									
Diameter (FtIn.)	Area (Sg. Ft.)	1.00	1.50	2.00	2.50	3.00	3.50						
5-0	19	0.125 (45)	0.100 (31)	0.100 (31)	0.100 (31)	0.100 (31)	0.100 (31)						
5-6	23	0.125-11-18	0.100	0.100	0.100	0.100	0.100						
6-0 6-6	28 32	(37) 0.125-II-18	(25) 0.100	(25) 0.100	(25) 0.100	(25) 0.100	(25) 0.100						
7-0	38	(32)	(22)	(22)	(22)	(22)	(22)						
7-6 8-0	44 50	0.125-II-9 (28)	0.150 (37)	0.100 (19)	0.100 (19)	0.100 (19)	0.100 (19)						
8-6	56	0.125-IV-9	0.125-11-18	0.100	0.100	0.100	0.100						
9-0 9-6	63 71	(25) 0.125-IV-9	(25) 0.125-II-18	(17) 0.125	(17) 0.100	(17) 0.100	(17) 0.100						
9-0 10-0	79	(22)	(22)	(22)	(15)	(15)	(15)						
10-6 11-0	87 95	0.175-IV-9 (32)	0.125-II-18 (20)	0.125-II-27 (20)	0.100 (14)	0.100 (14)	0.100 (14)						
11-6 12-0	104 114	(/	0.125-IV-18 (18)	0.125-II-27 (18)	0.125 (18)	0.100	0.100 (12)						
12-6	124		0.150-IV-18	0.125-11-27	0.150	0.125	0.125						
13-0 13-6	134 145		(23) 0.125-IV-9	(17) 0.125-IV-27	(23) 0.125-II-27	(17) 0.150	(17) 0.150						
14-0	156		(16)	(16)	(16)	(21)	(21)						
14-6 15-0	167 179		0.125-II-54 (15)	0.125-IV-9 (15)	0.125-IV-27 (15)	0.125-II-27 (15)	0.125-II-54 (15)						
15-6 16-0	191 204		0.150-IV-9 (18)	0.125-IV-18 (14)	0.125-II-27 (14)	0.150-II-54 (18)	0.150-II-54 (18)						
16-6 17-0	217 231		0.225-IV-9 (27)	0.150-IV-18 (17)	0.150-II-27 (17)	0.150-II-27 (17)	0.150-II-27 (17)						
17-6 18-0	245 259			0.175-IV-18 (19)	0.175-II-27 (19)	0.175-II-27 (19)	0.175-II-27 (19)						
18-6 19-0	27 4 289			0.175-IV-9 (18)	0.175-IV-27 (18)	0.175-II-27 (18)	0.175-II-27 (18)						
19-6	305			0.200-IV-9 (20)	0.200-IV-27 (20)	0.200-II-27 (20)	0.200-II-27 (20)						

^{*} Number in () below combination indicates maximum cover for the given combination plate thickness, rib type and rib spacing. All maxium cover depths are given in feet. (See Note Number 2 Under Structural Plate Notes Sheet 6 of 6).

MINIMUM AND MAXIMUM COVER FOR ALUMINUM STRUCTURAL PLATE

LAST REVISION 07/01/07



4. Tables based on HS 20 wheel loads. LAST

REVISION

07/01/07

Aluminum Structural Plate Height of Cover Limits* Combination Metal Thickness, Reinforcing Rib Type, and Rib Spacing Underpass Shape- HS 20 Live Load

			Minimum Height of Cover (Ft.)					
Span (FtIn.)	Rise (FtIn.)	Area (Sq.Ft.)	1.00	1.50	2.00	2.50	3.00	3.50
6-1	5-9	28	0.125-II-18 (29)	0.100 (25)	0.100 (25)	0.100 (25)	0.100 (25)	0.100 (25)
6-3 6-3 6-2 6-4 6-3 6-5	6-1 6-5 6-11 7-3 7-9 8-1	30 32 34 37 39 42	0.125-11-18 (25)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)
12-1	11-0	106		0.125-IV-18 (14)	0.125-II-27 (14)	0.125 (14)	0.100 (12)	0.100 (12)
12-10 13-0	11-2 12-0	114 124		0.150-IV-18 (13)	0.125-II-27 (13)	0.150 (13)	0.125 (13)	0.125 (13)
13-8 14-0	12-4 12-11	133 143		0.125-IV-9 (12)	0.125-IV-27 (12)	0.125-II-27 (12)	0.125-II-54 (12)	0.125-II-54 (12)
14-6 14-8	13-5 14-1	155 165		0.125-IV-9 (11)	0.125-IV-27 (11)	0.125-II-27 (11)	0.125-II-54 (11)	0.125-II-54 (11)
15-5 15-6	14-5 15-2	177 190		0.150-IV-9 (11)	0.125-IV-18 (11)	0.125-II-27 (11)	0.125-II-27 (11)	0.125-II-2 (11)
16-2 16-6 16-8	15-6 16-0 16-4	200 208 215		0.225-IV-9 (10)	0.150-IV-18 (10)	0.150-II-27 (10)	0.150-II-27 (10)	0.150-II-27 (10)

ALUMINUM STRUCTURAL PLATE NOTES

DESCRIPTION:

- 1. Allowable cover (minimum & maximum) is measured from the outside valley of crown plate to the bottom of flexible pavement or from the outside valley of the crown plate to the top of rigid pavement. Minimum cover must be maintained in unpaved areas. Maximum cover is measured at the highest fill and/or the highest pavement elevation.
- 2. To find the minimum material requirements for the aluminum structural plate structure:
- a. Select the span in the left hand column that is equal to or larger than structure size required.
 - b. Select the cover in the top row that is equal to or smaller than that required for the site.
 - c. Intersect appropriate span and cover to find the appropriate plate. Example: Round Pipe, Span= 17'-0", Height of Cover= 2'-7" (use 2.5 ft. in table). Ans: 0.150-II-27 (17) The table selections show metal thickness, rib type, rib spacing and maximum cover. Example: 0.150-II-27=0.150" thick plate structure with Type II rib at 27" on centers on the crown. Number (17) in parenthesis below combination indicates maximum cover in feet for the given combination of plate thickness, rib type, and rib spacing.
- 3. Arch shapes shown are single radius and have a rise-to-span ratio of 0.30 to 0.53 Structures with rise-to-span ratios of less than 0.30 are typically not used because of structural considerations.

Aluminum Structural Plate Height of Cover Limits* Combination Metal Thickness, Reinforcing Rib Type, and Rib Spacing Pipe-Arch Shape- HS 20 Live Load

			Minimum Height of Cover (Ft.)						
Span (FtIn.)	Rise (Ft-In)	Area (Sq-Ft)	1.00	1.50	2.00	2.50	3.00	3.50	
6-7 6-11	5-8 5-9	30 32	0.125-II-18 (25)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)	0.100 (22)	
7-3 7-9 8-1	5-11 6-0 6-1	34 37 39	0.125-IV-18 (22)	0.150 (22)	0.100 (19)	0.100 (19)	0.100 (19)	0.100 (19)	
8-5 8-10	6-3 6-4	42 45	0.125-IV-9 (19)	0.125-II-18 (19)	0.100 (17)	0.100 (17)	0.100 (17)	0.100 (17)	
9-3 9-7 9-11	6-5 6-6 6-8	47 50 53	0.125-IV-9 (17)	0.125-II-18 (17)	0.125 (17)	0.100 (15)	0.100 (15)	0.100 (15)	
10-3 10-9 11-1	6-9 6-10 7-0	56 58 61	0.175-IV-9 (16)	0.125-II-18 (16)	0.125-II-27 (16)	0.100 (14)	0.100 (14)	0.100 (14)	
11-5 11-9	7-1 7-2	64 68		0.125-II-18 (14)	0.125-II-27 (14)	0.125 (14)	0.100 (12)	0.100 (12)	
12-3 12-7 12-11 13-1 13-1	7-3 7-5 7-6 8-2 8-4	71 74 77 83 87		0.150-IV-18 (13)	0.125-II-27 (13)	0.150 (13)	0.100 (11)	0.100 (11)	
13-11 14-0 13-11	8-5 8-7 9-5	90 94 102		0.125-IV-9 (12)	0.125-IV-27 (12)	0.125-II-27 (12)	0.100 (11)	0.100 (11)	
14-3 14-8 14-11	9-7 9-8 9-10	106 110 114		0.125-IV-9 (11)	0.125-IV-27 (11)	0.125-II-27 (11)	0.125 (11)	0.125 (11)	
15-4 15-7 16-1	10-0 10-2 10-4	119 123 128		0.150-IV-9 (11)	0.125-IV-18 (11)	0.125-II-27 (11)	0.125-II-54 (11)	0.125 (11)	
16-4	10-6	132		0.225-IV-9 (10)	0.150-IV-18 (10)	0.125-II-27 (10)	0.125-II-54 (10)	0.125-II-54 (10)	

^{*} Number in () below combination indicates maximum cover for the given combination plate thickness, rib type and rib spacing. All maximum cover depths are given in feet. (See Note Number 2 Under Structural Plate Notes)

DESIGN NOTES

- 1. The plans must call out size, metal thickness, reinforcing rib type and rib spacing.
- 2. Pipe-arch and underpass shapes will generate high corner bearing pressures against the sidefill and foundation. The height of cover is directly affected by these bearing pressures. The surrounding soil and foundation must be checked to ensure that they to react against these pressures to avoid inducing excessive strain in plate.

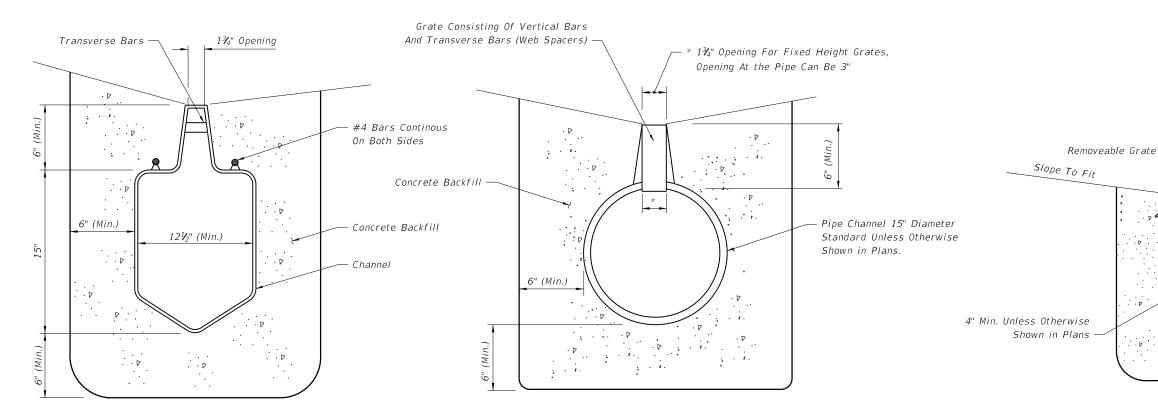
MINIMUM AND MAXIMUM COVER FOR ALUMINUM STRUCTURAL PLATE



IAST

REVISION





PREFORMED CHANNEL WITH REMOVABLE GRATE

SEE SHEET 2 FOR TYPICAL LOCATIONS

TYPE II

DESIGN NOTES

- 1. Where placed adjacent to reinforced concrete barrier wall or median barrier wall, the designer shall detail in the plans the position of the drain relative to the barrier wall to avoid conflicts with the barrier wall footing. See Index No. 410.
- 2. The designer shall identify the following in the plans:
 - (a) The type of drain at each location.
 - (b) The begin and end locations of the Trench Drain.
 - (c) The location of the outlet pipe if the Trench Drain is not stubbed directly into a drainage structure.
 - (d) The design flow (Q) for the Trench Drain must be shown on the plans.
- 3. Capture efficiency for Type I Trench Drain may be computed using the equations for slotted drain in FHWA's HEC 12 & 22. Grate Type I and Type II must have at least 30% open area.
- 4. Round pipe alternate is available in 12, 18, 24 and 36 inch.
- 5. Type II Preformed Channel with integral anchoring lugs are applicable.

SEE SHEET 2 FOR TYPICAL LOCATIONS

TYPE I (NON-REMOVABLE GRATE)

GENERAL NOTES

PREFORMED POLYETHYLENE ALTERNATE

- 1. Trench drain is intended for use in gutters and driveways as shown on the typical locations on Sheet 2. Type 1 is intended for use in Type E, F and drop curbing, and adjacent to traffic separators and standard barrier walls. The width of the channel grate for Type I Trench Drain shall be 1¾" throughout varying the depth of the channel neck. Type II may also be used in those locations if an independent laboratory certifies that the grating used has an open area equal to at least 0.27 square feet per linear foot. Type II is primarily intended for use in valley gutter across driveway openings and drop curbing; Type I may also be used in those locations. The width of the channel grate for Type II Trench Drain shall be the same as the width of the channel. The linear slope or gradient for Type II may be manufactured by varying the depth of the channel. Trench Drain shall not be placed in pedestrian paths unless ADA compliant grates are used.
- 2. Unless shown in the plans, outlet pipes and preformed channel inverts shall be sloped 0.6% or steeper toward the outlet regardless of the surface slope.
- 3. Trench drain may be stubbed directly into drainage structures, or outlet pipes may be used to connect trench drain to drainage structures.
- 4. A cleanout port compatible with the manufactured system shall be provided for Type I drains at the upstream end and at intervals not to exceed 50 feet. The cleanout port shall provide an opening 6" to 10" wide (transverse to the trench drain length) and 18" to 24" long. Where cleanouts are placed adjacent to raised curb or separator, the curb or separator shall be formed around the cleanout. The cleanout shall have a removable load resistant cover or grate.
- 5. Trench excavation must allow for a minimum of 6" of concrete to be placed under and alongside the trench drain channel system. Concrete backfill shall meet the requirements of Section 347 of the Standard Specifications. At the end of all units (Type I or II), the concrete backfill shall extend 6" minimum past the end of the drain opening.
- 6. Transverse bars for Type I Trench Drain shall be spaced 4" to 6" on center.
- 7. Whenever the work disturbs existing conditions or work already completed, restore the same to its original condition in every detail. All such repair and replacement shall meet the approval of the Engineer.
- 8. Payment to be made under the contract unit price for Trench Drain, LF.

ROUND ALTERNATE

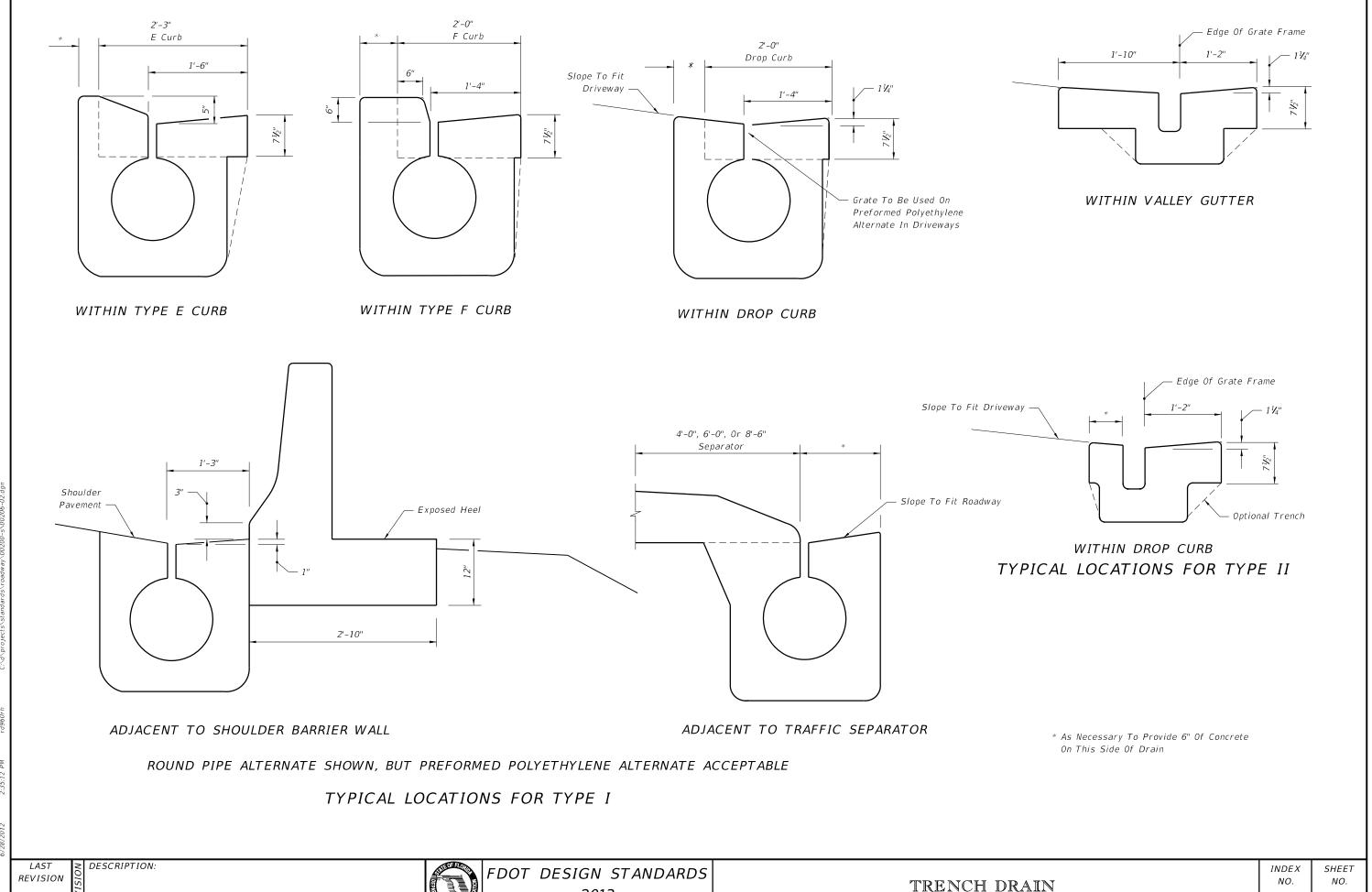
- Grate Frame With Anchoring Lug

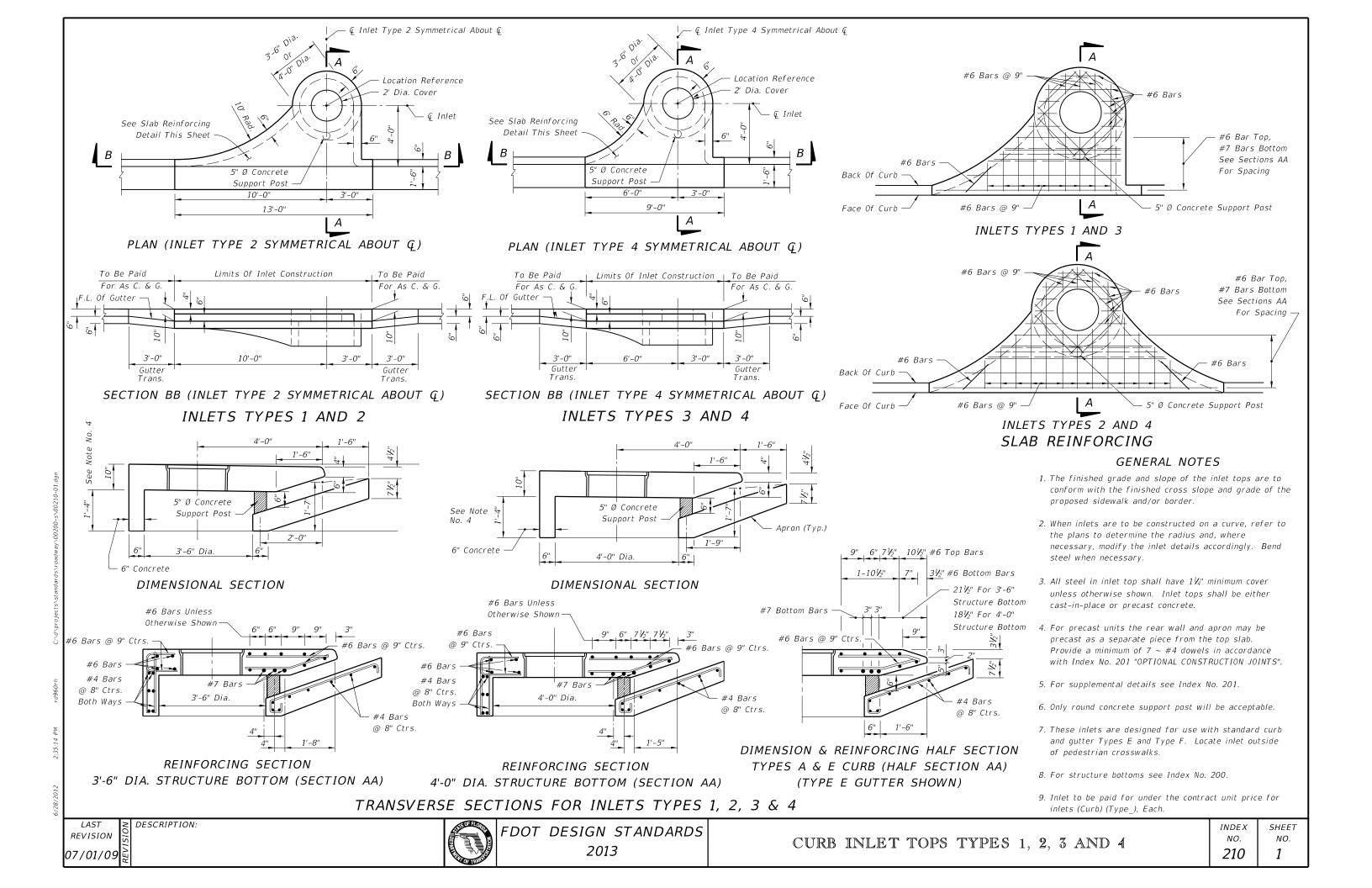
Drain Channel

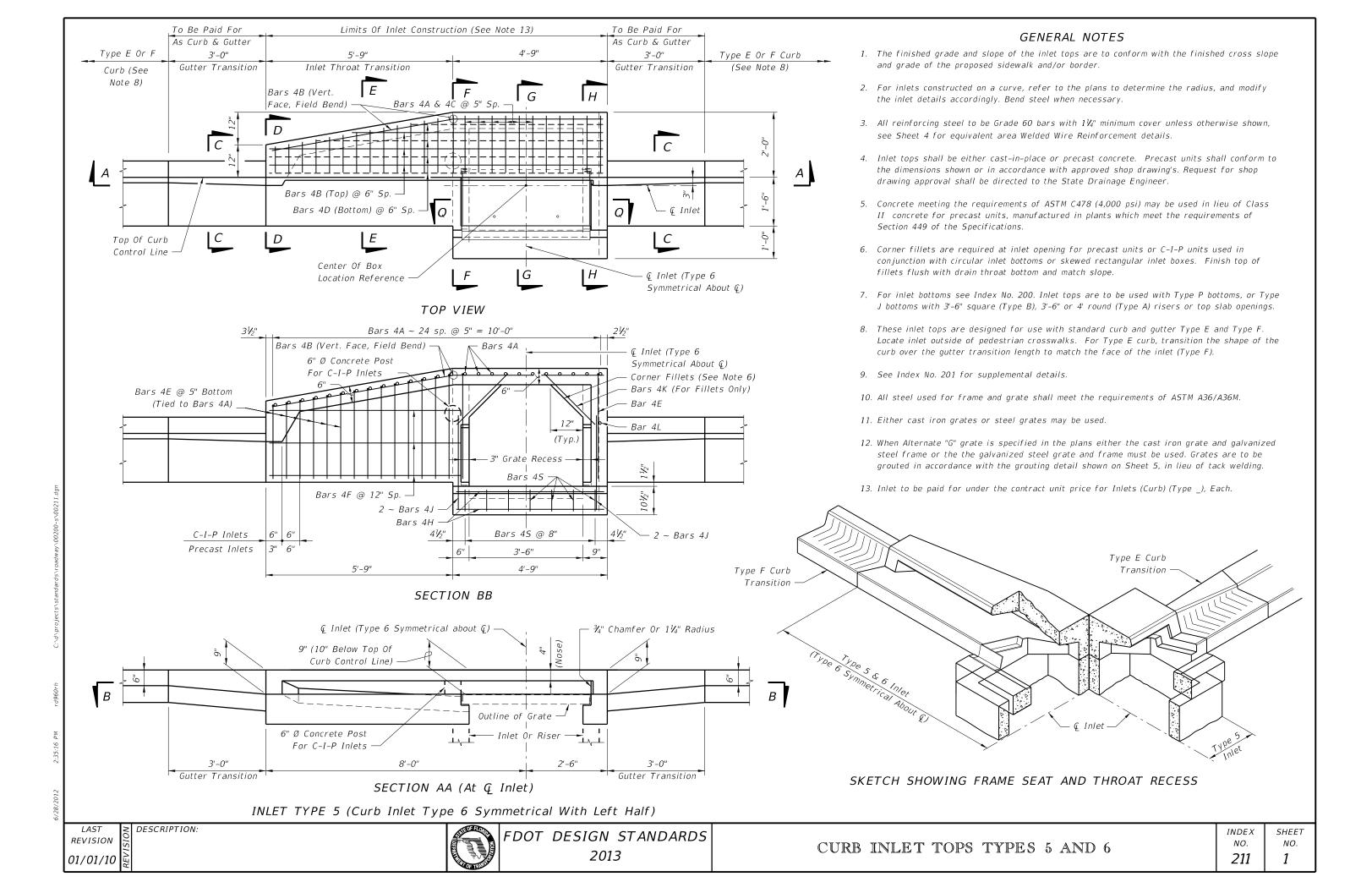
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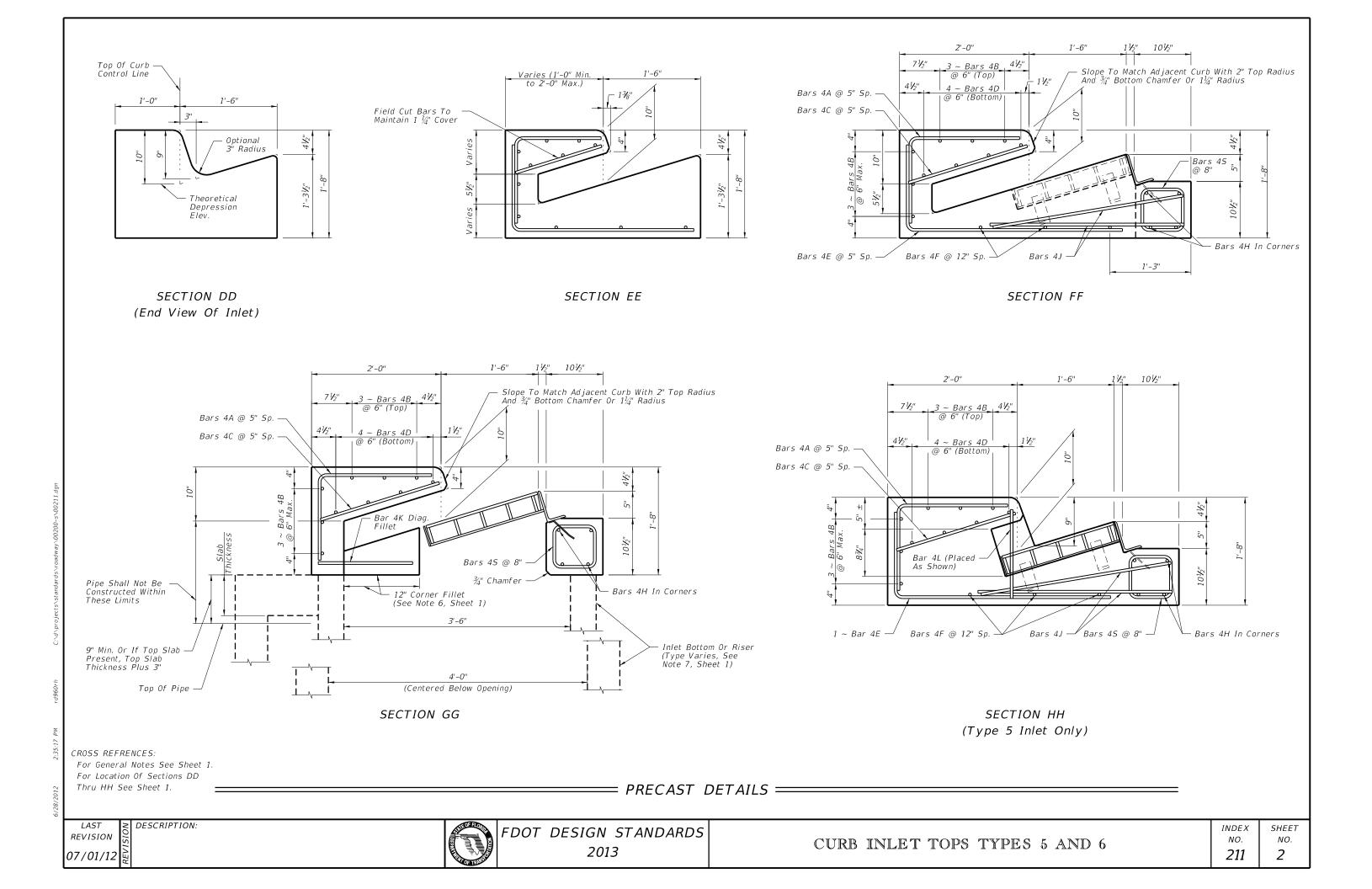
Drain Channel

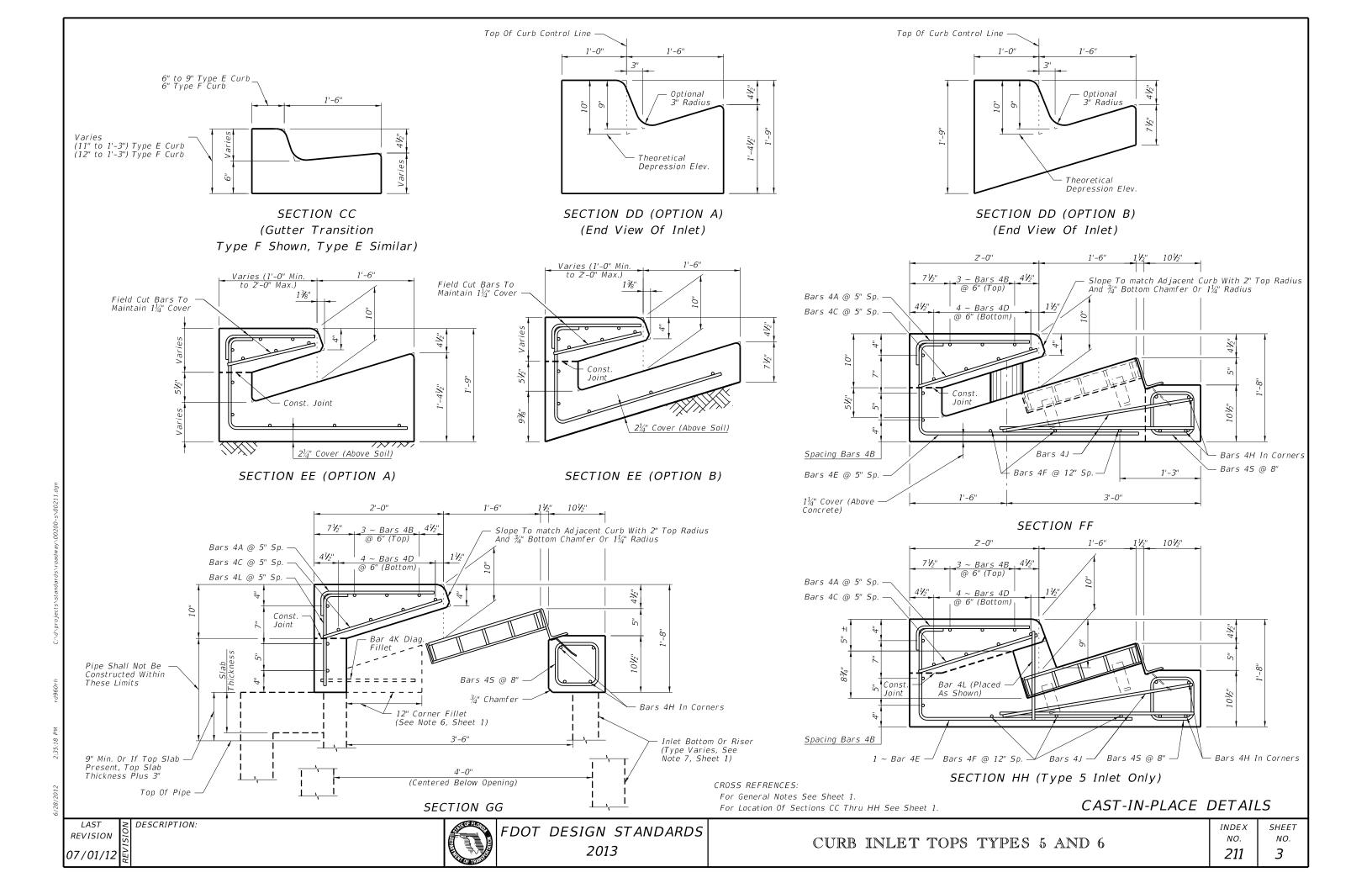
(Bottom Shape Optional)

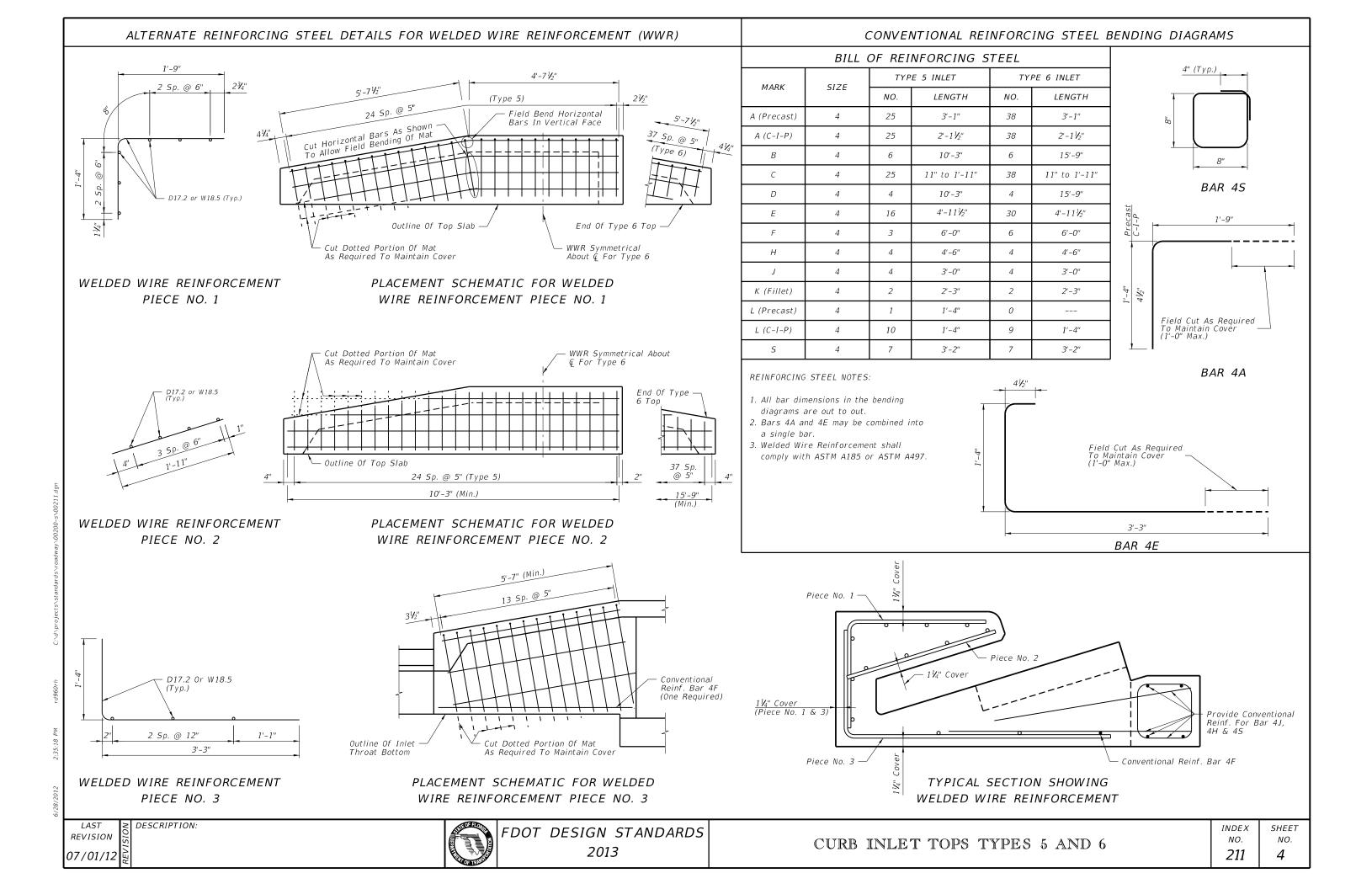


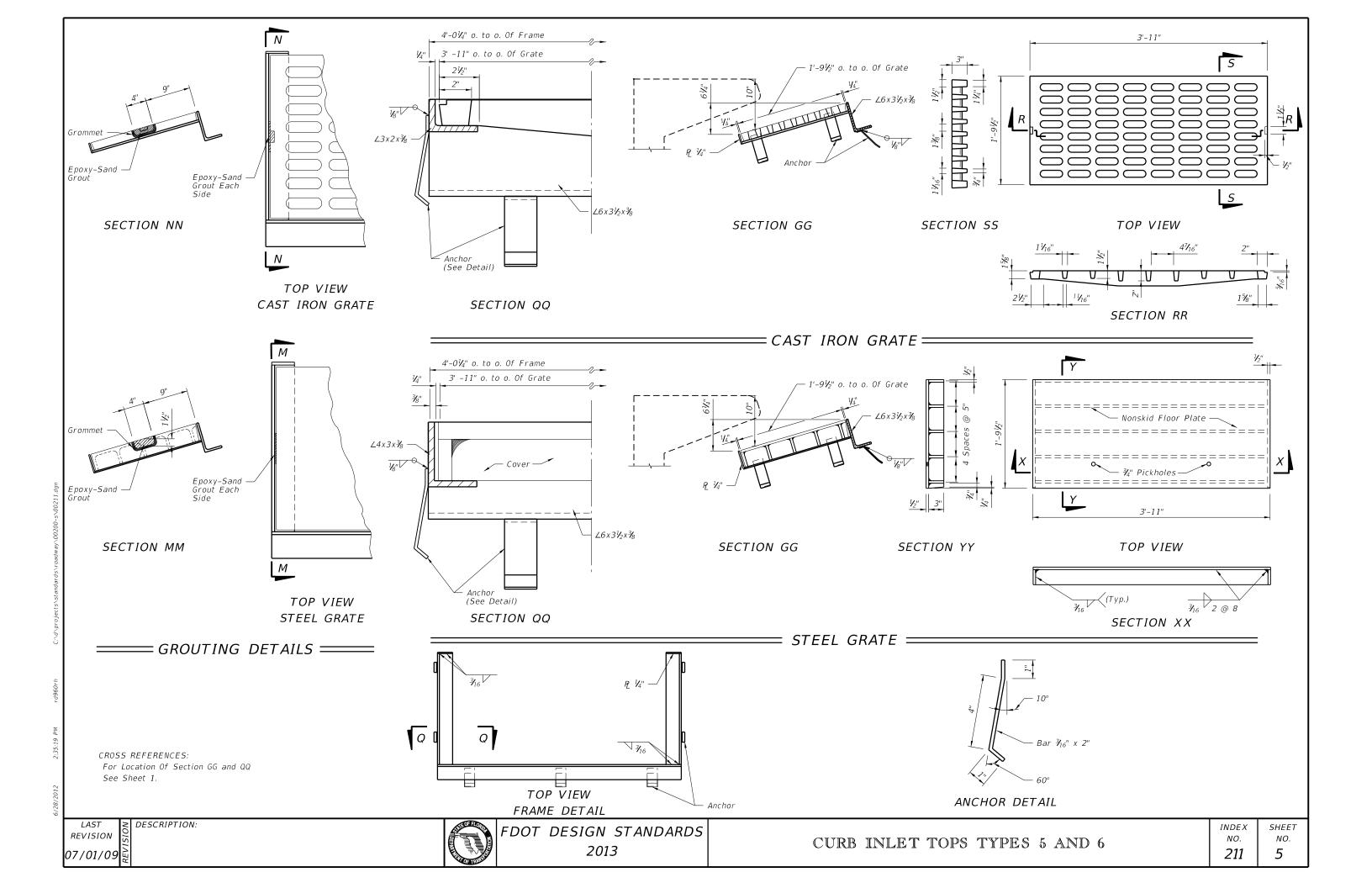


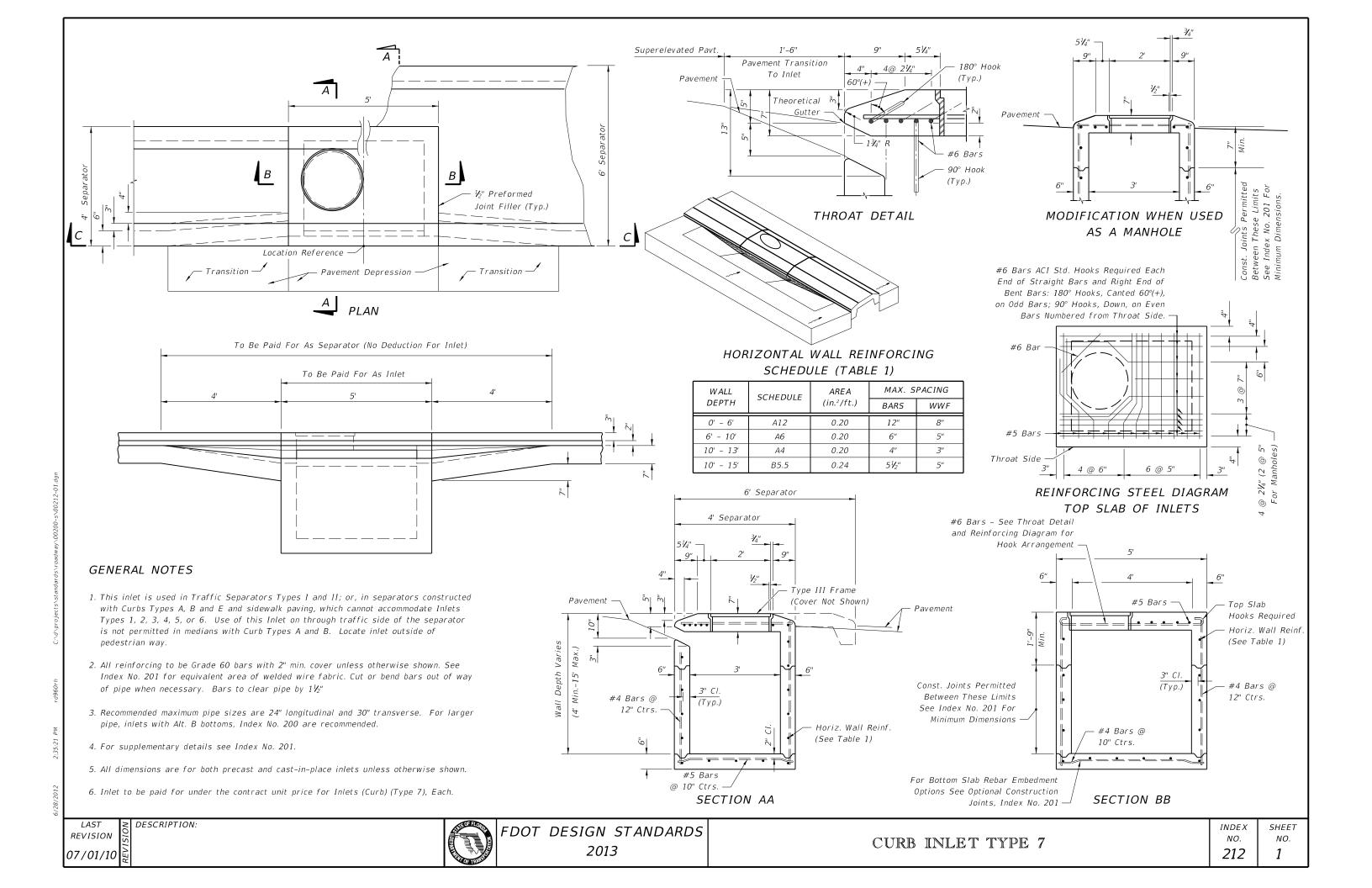


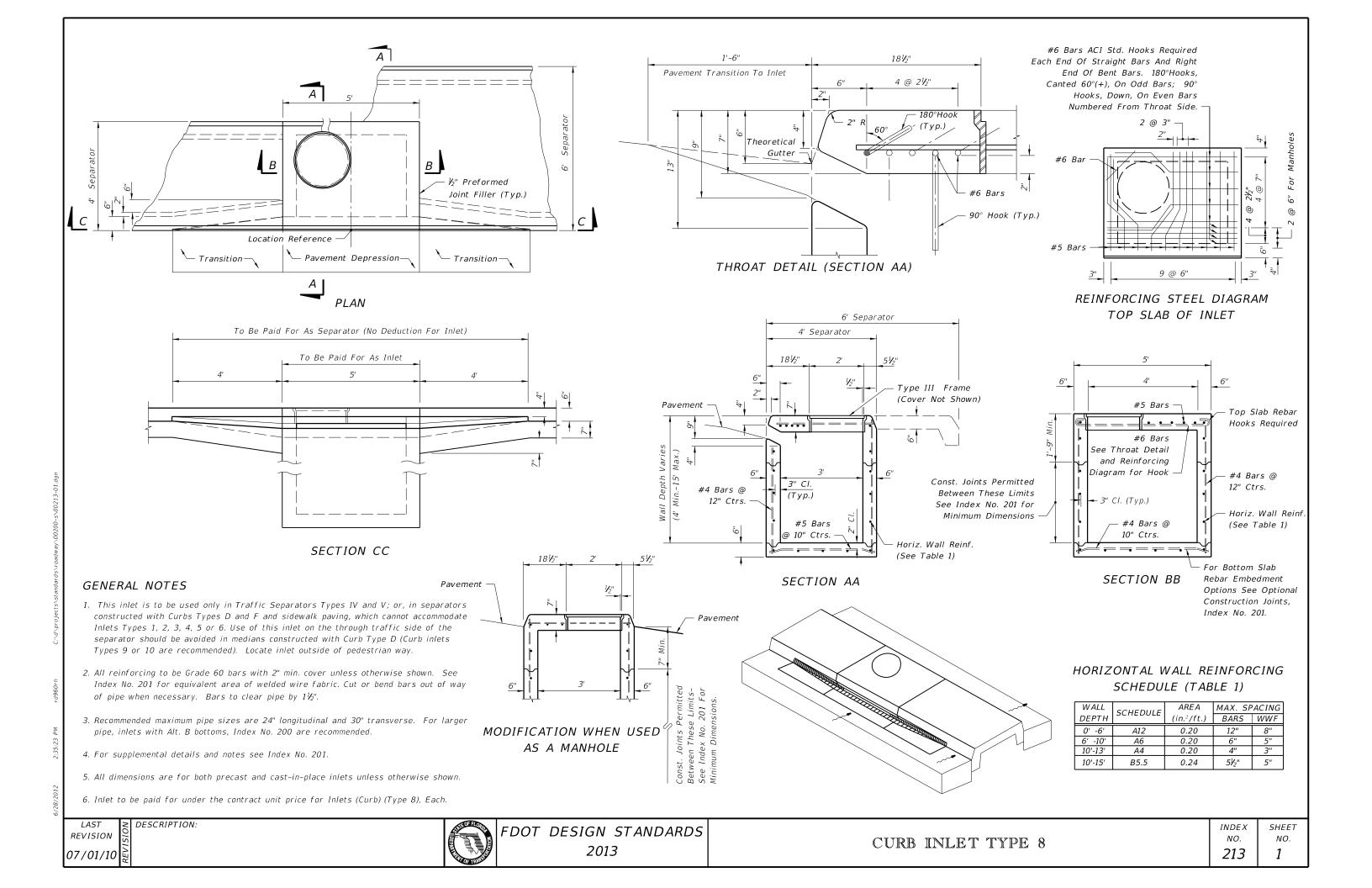


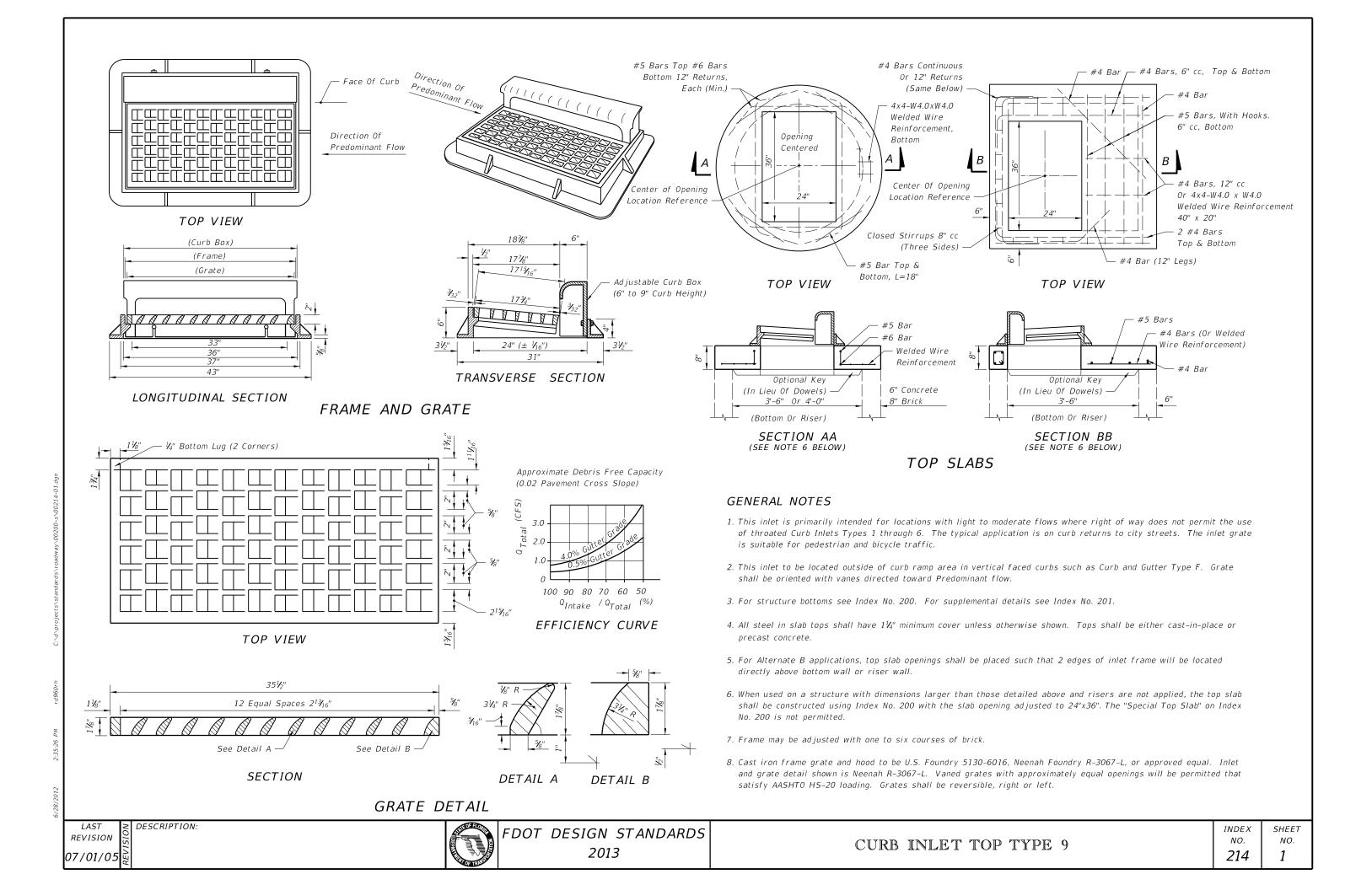


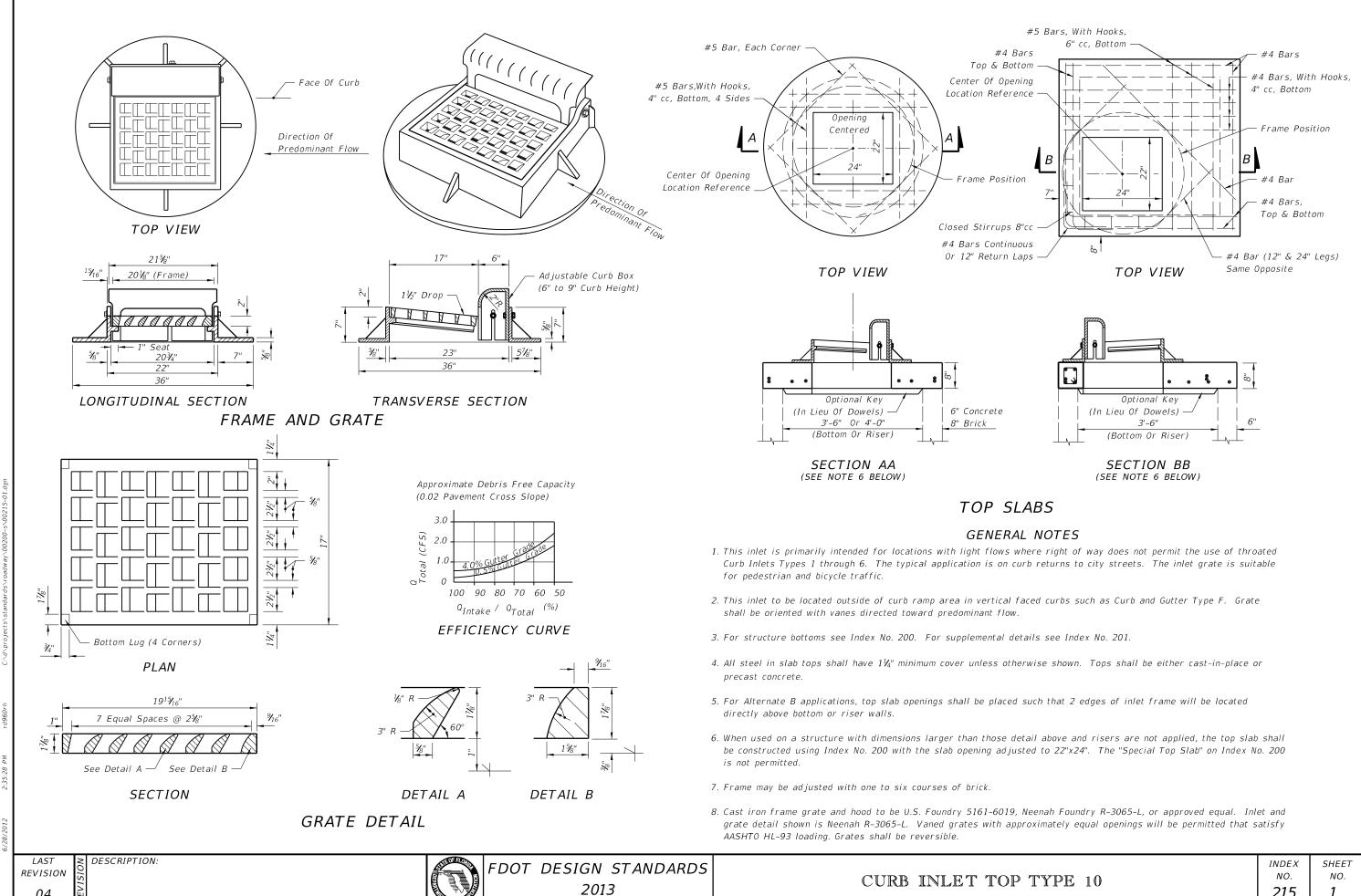


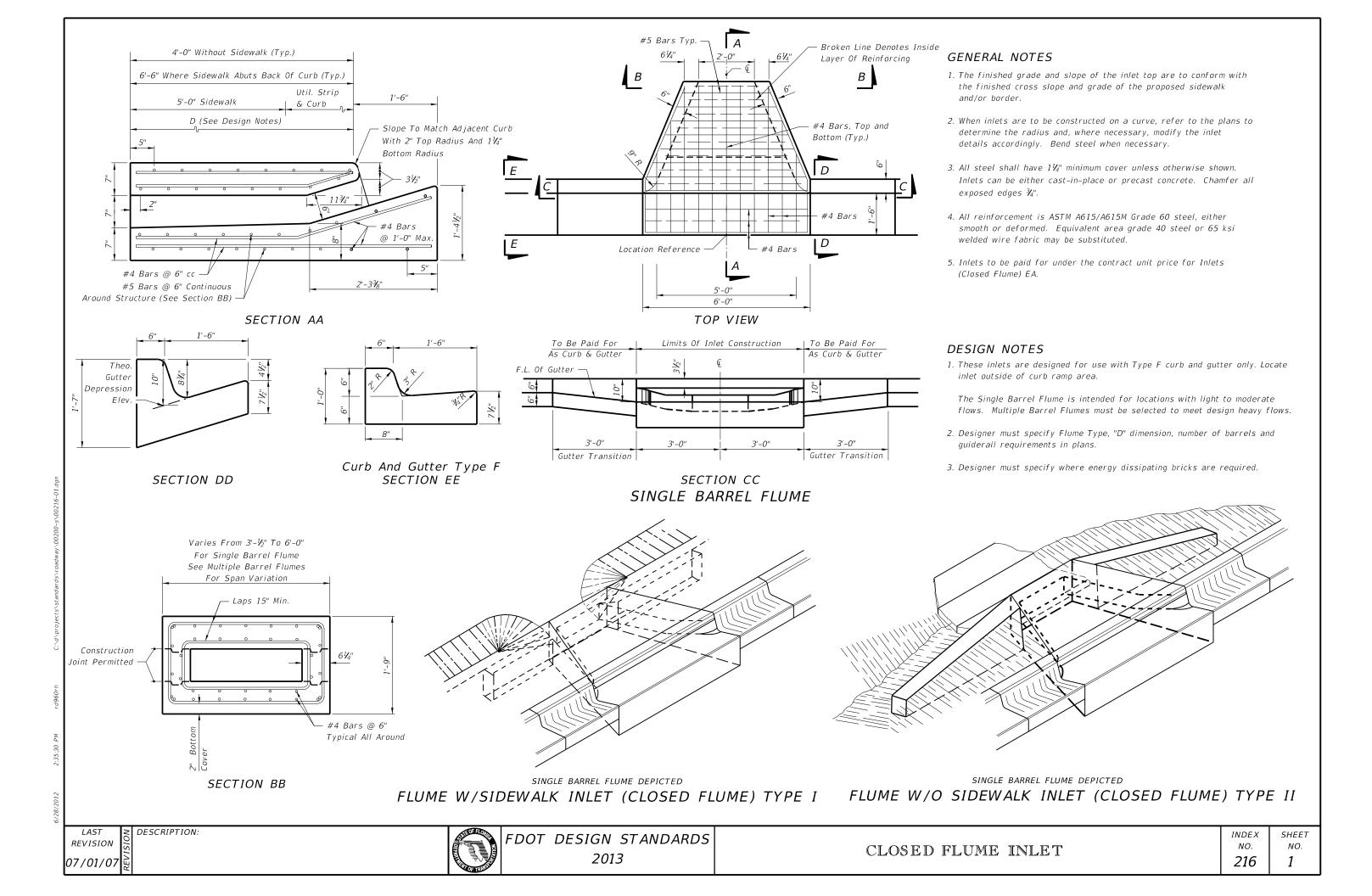


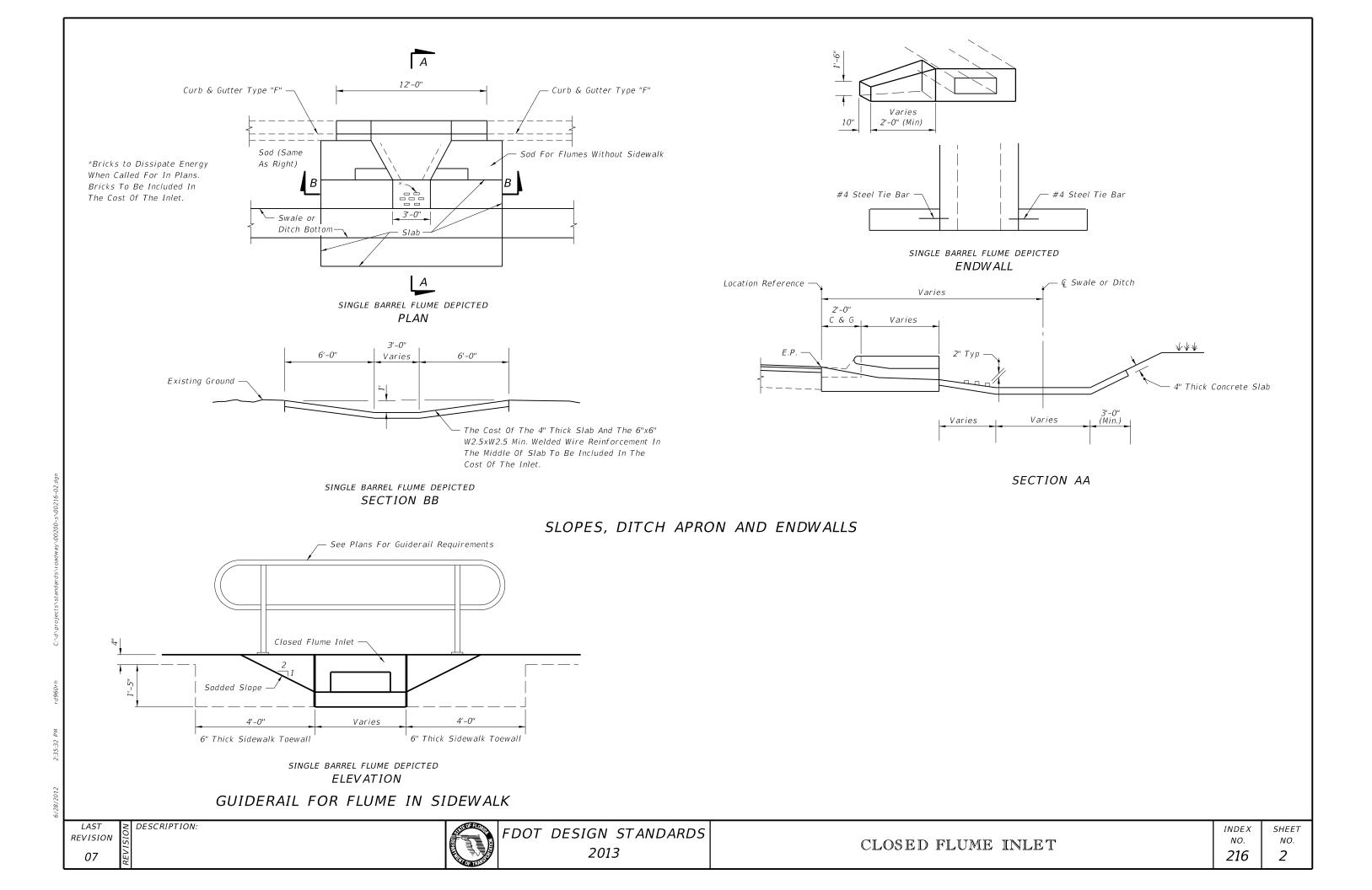


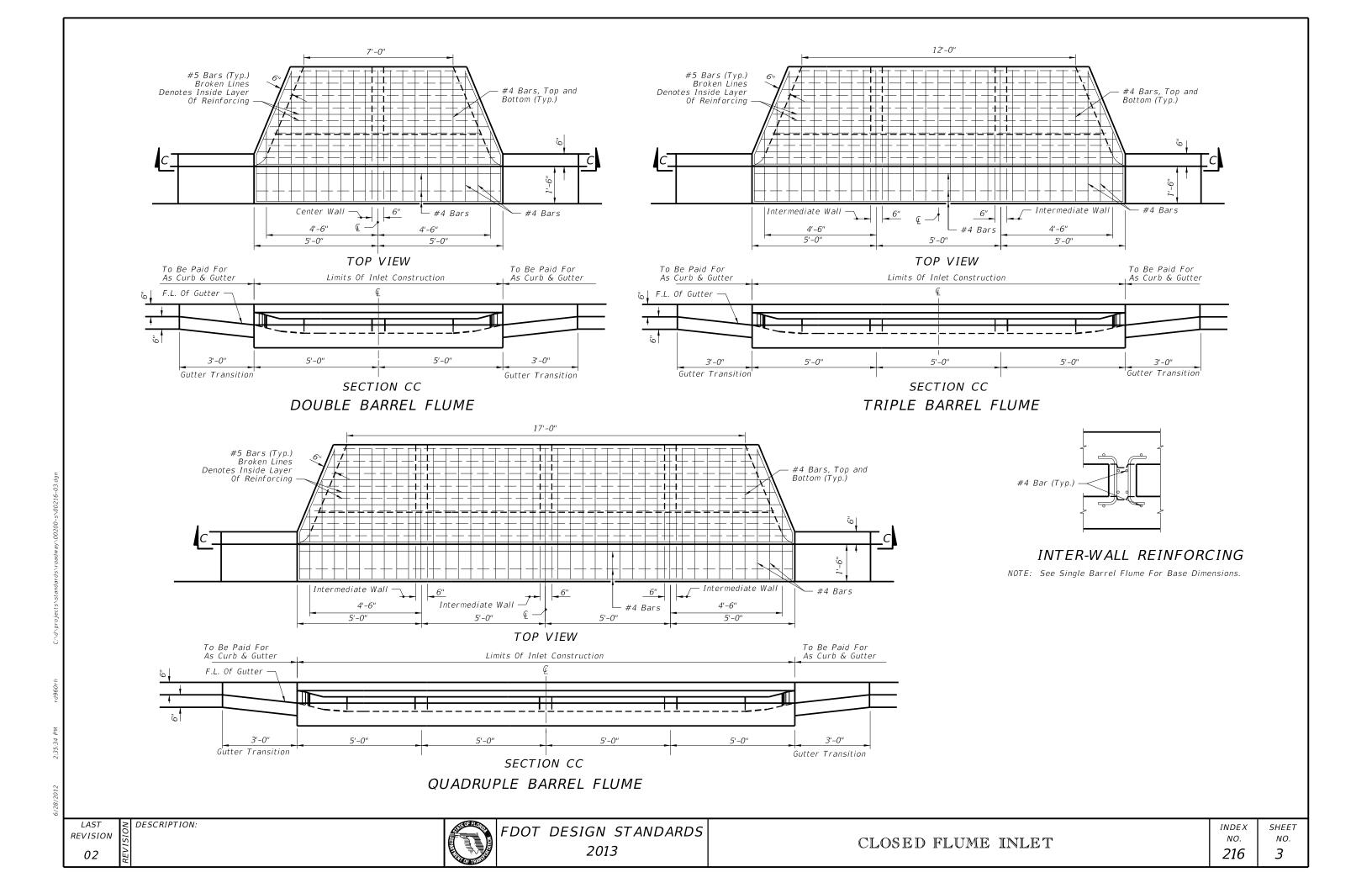


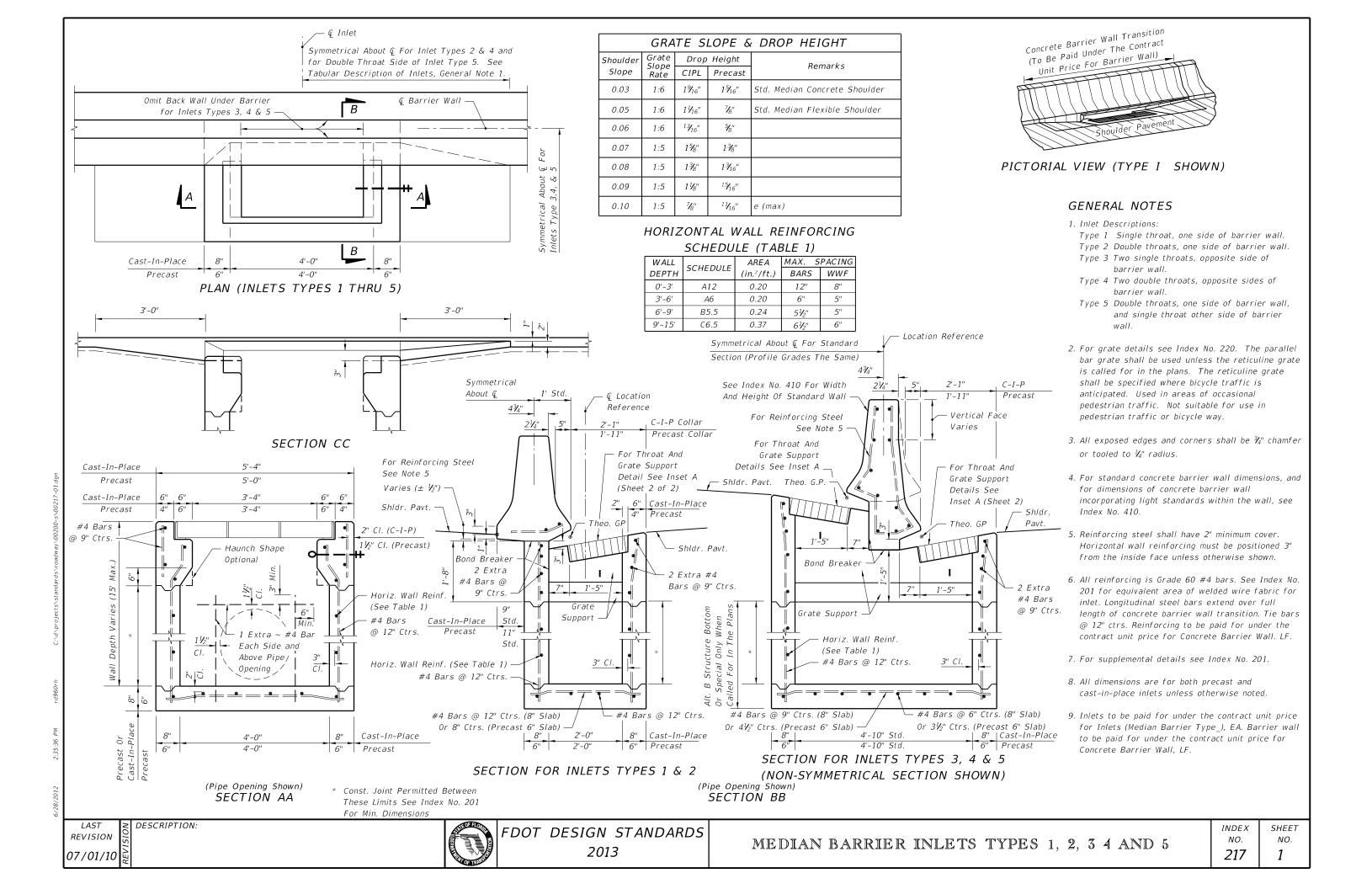


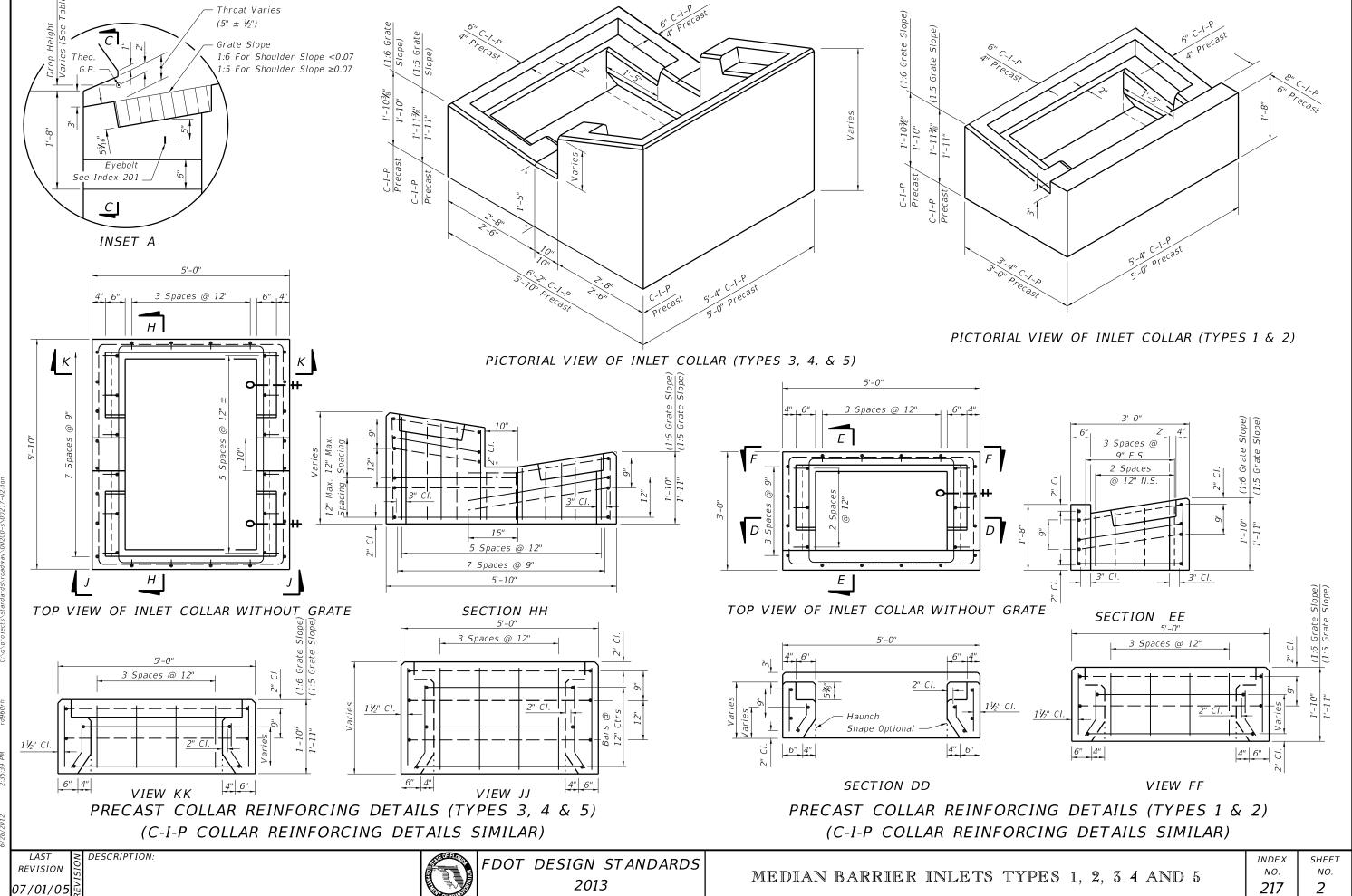




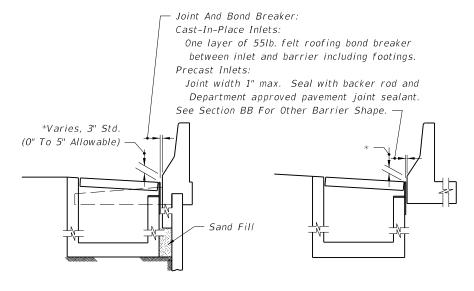




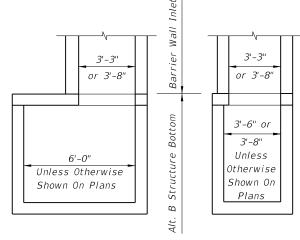




Concrete Barrier Wall

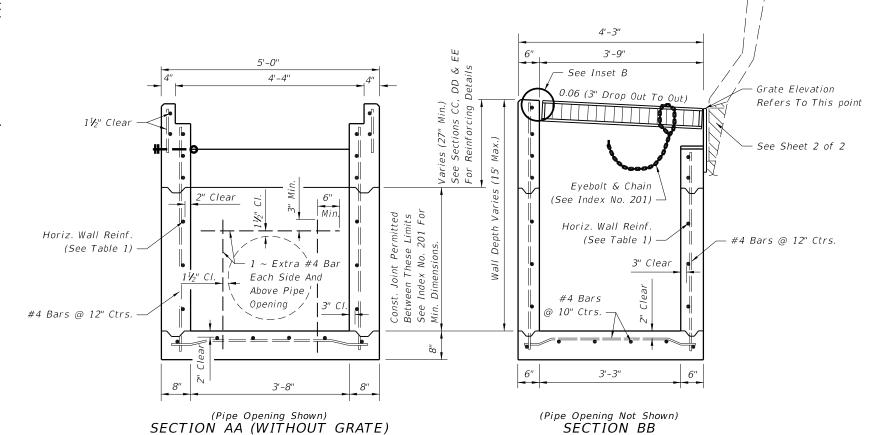


BARRIER WALL / RETAINING WALL SINGLE FACE ROADWAY BARRIER INLET SECTION AT WALLS



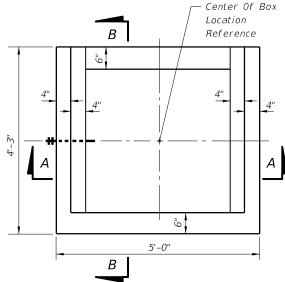
Note: Alt. B Structure Bottom Only. See Index No. 200.

INLET WITH STRUCTURE BOTTOM



GENERAL NOTES

- 1. This inlet is primarily intended for use adjacent to concrete barrier walls on paved shoulders. Use of the inlet adjacent to other wall types shall be approved by the Drainage Engineer. The inlet is suitable for bicycle and occasional pedestrian traffic, with roller bar installation (see inset B) but should not be placed in a designated pedestrian travel way. It is not intended for use in curb and gutter or other areas where throated inlets are required, nor areas subject to high debris.
- 2. Inlets located in embankments constructed with earth anchored retaining wall shall be designed with minimum depths to reduce adverse impact on the anchorage system. Runs of pipe parallel to and near anchored wall shall be avoided wherever practical. Special coordination must be exercised during the design and construction of storm water systems within anchored wall systems.
- 3. Inlet bottoms and/or tops may be either precast or cast-in-place. Whether cast as a single unit or as multiple segments, and whether precast or cast-in-place, the upper 2'-3" of the inlet shall be reinforced in accordance with sections CC, DD and EE.
- 4. All exposed edges and corners shall be $rac{3}{4}$ " chamfer or tooled to $rac{1}{4}$ " radius.
- 5. When Alternate G grate is specified in the plans, the grate is to be hot-dip galvanized after fabrication. Field installation of the filler bar called for in Inset B will not be permitted, thereby requiring tolerance adjustment during fabrication and/or casting, or, matching grate to structure prior to galvanizing.
- 6. All reinforcing is Grade 60 bars. See Index No. 201 for equivalent area of welded wire fabric.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 8. For supplemental details see Index Nos. 200 and 201.
- 9. Inlets to be paid for under the contract unit for Inlets (Barrier Wall), Each.



TOP VIEW (WITHOUT GRATE)

HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

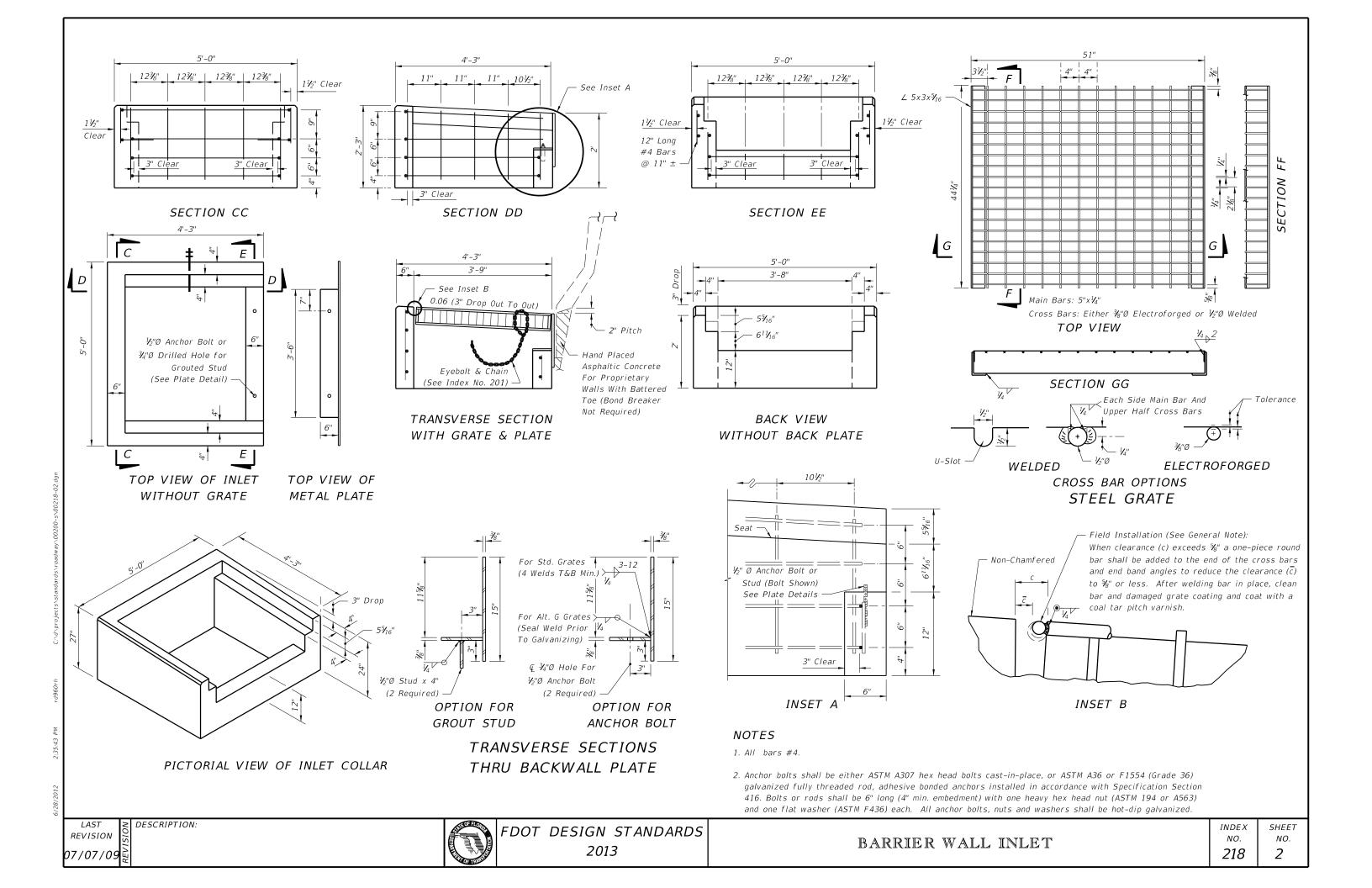
WALL	SCHEDULE	AREA	MAX. S	MAX. SPACING		
DEPTH	JCHLDULL	(in.²/ft.)		WWF		
0'-5'	A12	0.20	12"	8"		
5'-10'	A6	0.20	6"	5"		
10'-15'	A4	0.20	4"	3"		
10'-15'	B5.5	0.24	5½"	5"		

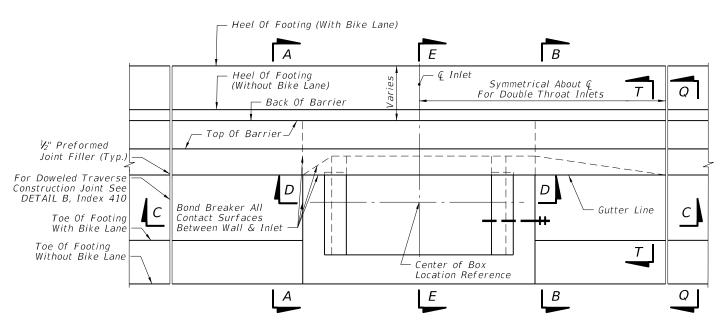
LAST REVISION 07/01/10

DESCRIPTION:

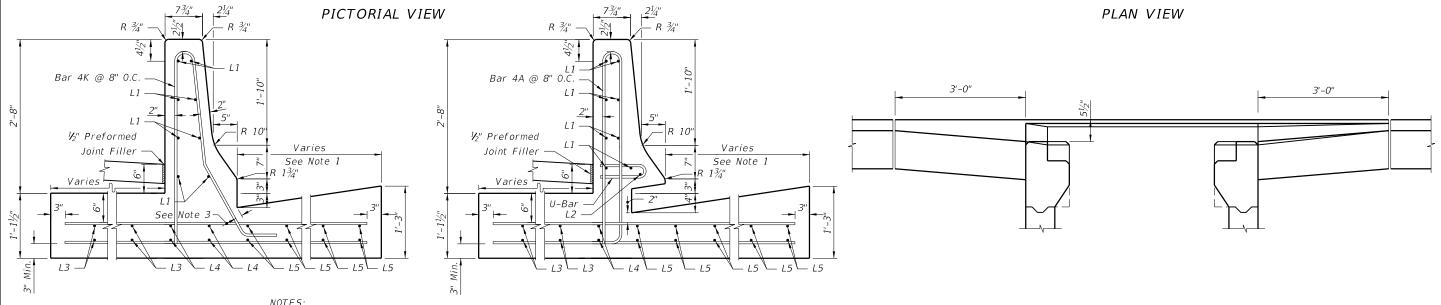


DOT DESIGN STANDARDS 2013





NOTE: For Sections QQ & TT, see Index 410.



- Distance = 1'-6" With Bike Lane, Or 2'-6" Without Bike Lane.
- For Double Throat Inlet, Section BB replaces Section AA. 3. Field Bend Bar 4K To Maintain 2" Minimum Concrete Cover.

SECTION AA SECTION BB

GENERAL NOTES

- 1. This inlet to be used in conjunction with Concrete Barrier Wall, Curb and Gutter, Index No. 410. The inlet is suitable for bicycle and occasional pedestrian traffic with extended crossbar or bar stub (see INSETS B & B ALTERNATE). Inlet should not be placed in a pedestrian way.
- 2. All reinforcing is Grade 60 bars. For equivalent area of welded wire fabric for inlet, see Index No. 201. Reinforcing shall have 2" min. cover unless otherwise shown. Bar's shall be trimmed or bent to provide 11/2" clearance around pipe openings. Cost for additional reinforcing in barrier wall to be included in cost of concrete barrier wall.
- 3. Barrier wall shall be Class II Concrete, finished surface in accordance with General Note 1, Sheet 1, Index No. 410.
- 4. All exposed edges and corners shall be 3/4" chamfer or tooled to 1/4" radius.
- A flat $18" \times 2\frac{1}{9}"$ drainage slot shall be constructed at the inlet centerline when the inlet is located in a curb sag. For drainage slot construction, no more than two bars shall be trimmed or deleted such as type: 4A, 4K, and U-Bar. On each side of drainage slots, vertical & horizontal bars shall be placed to provide 2" concrete cover.

- 6. Recommended maximum pipe sizes are 18" longitudinal and 30" transverse. For larger pipe, use Alternate B bottoms, Index No. 200.
- Grates can be fabricated with reticuline bars or with either 1/2"○ welded or 3/8"○ electroforged cross bars and bearing bars as detailed on Sheet 3.
- When Alternate G grate is specified in plans, the grate is to be hot-dip galvanized after fabrication according to Specification 962-9.

SECTION CC

- For Pay Item purposes, the depth of the barrier wall inlet shall be computed using the center of box grate elevation, less the flow line elevation of the lowest pipe flow line or to the top of the sump floor elevation.
- 10. All dimensions are for both precast and C-I-P inlets unless otherwise noted.
- 11. Inlets to be paid for under the contract unit price for Inlets, Barrier Wall, Rigid, Curb & Gutter, Each.
- 12. Concrete Barrier Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall, Rigid-Curb & Gutter, LF.

LAST REVISION 01/01/12

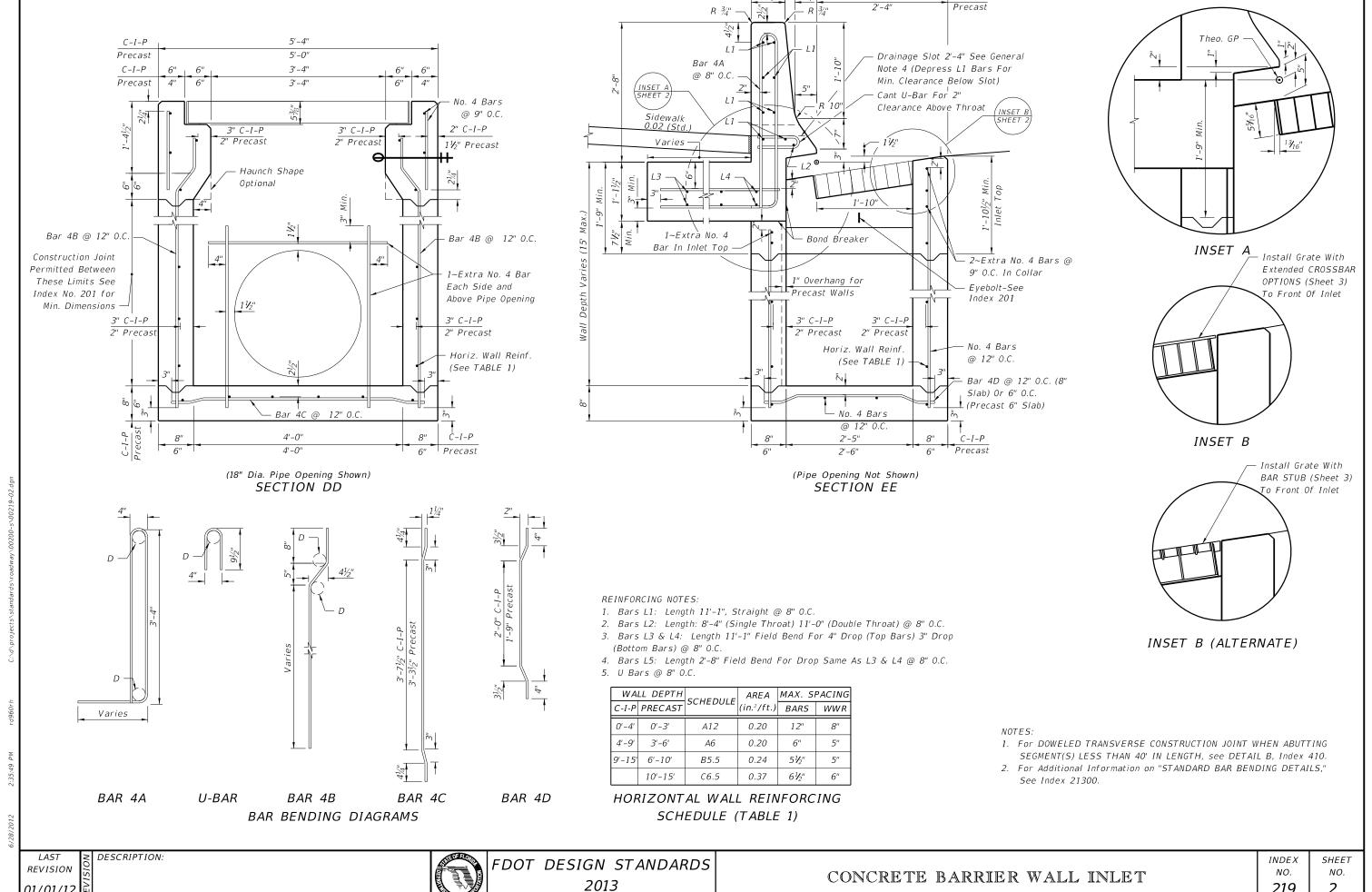
DESCRIPTION:

FDOT DESIGN STANDARDS 2013

CONCRETE BARRIER WALL INLET

INDEX SHEET NO. 219

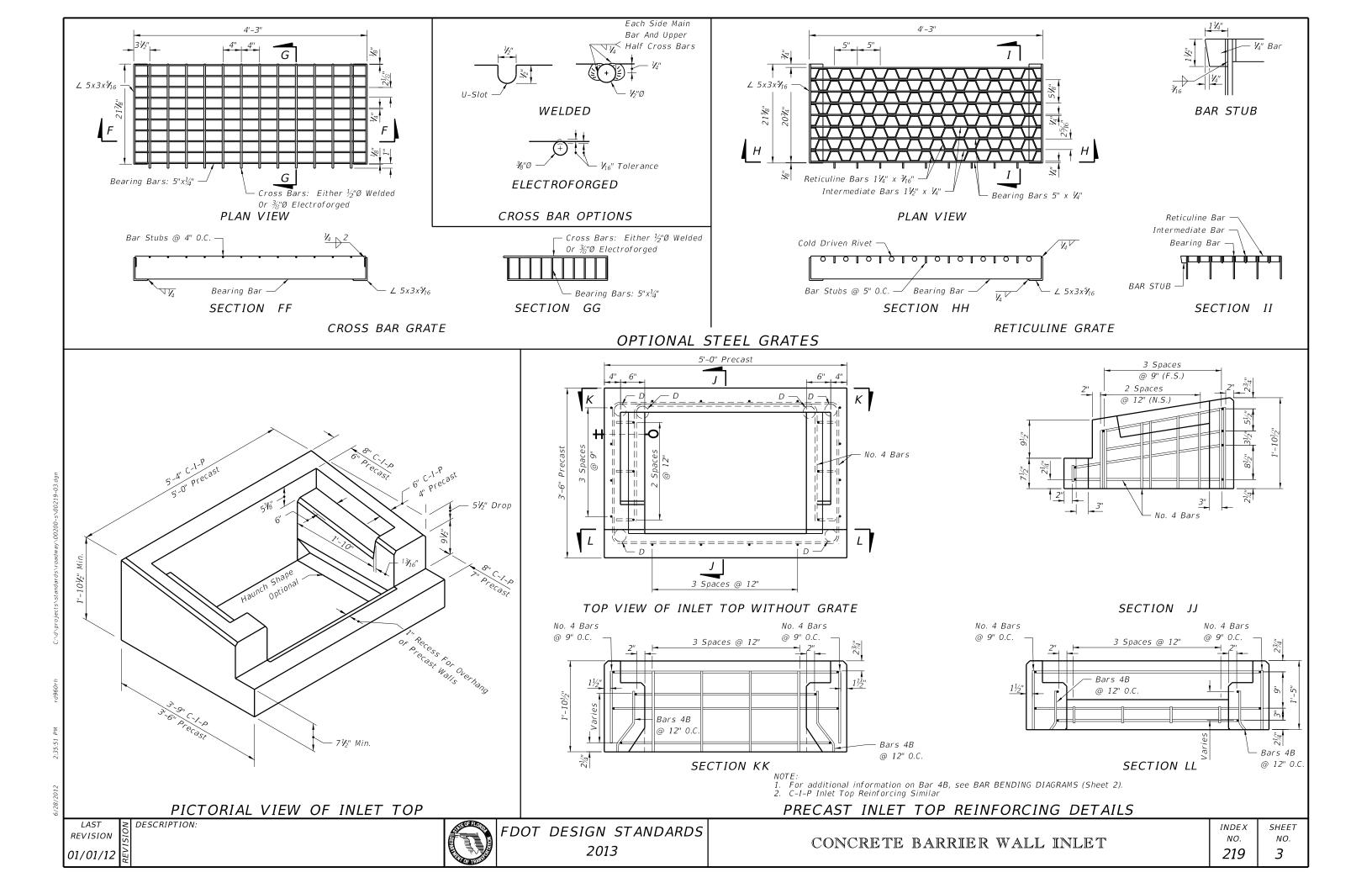
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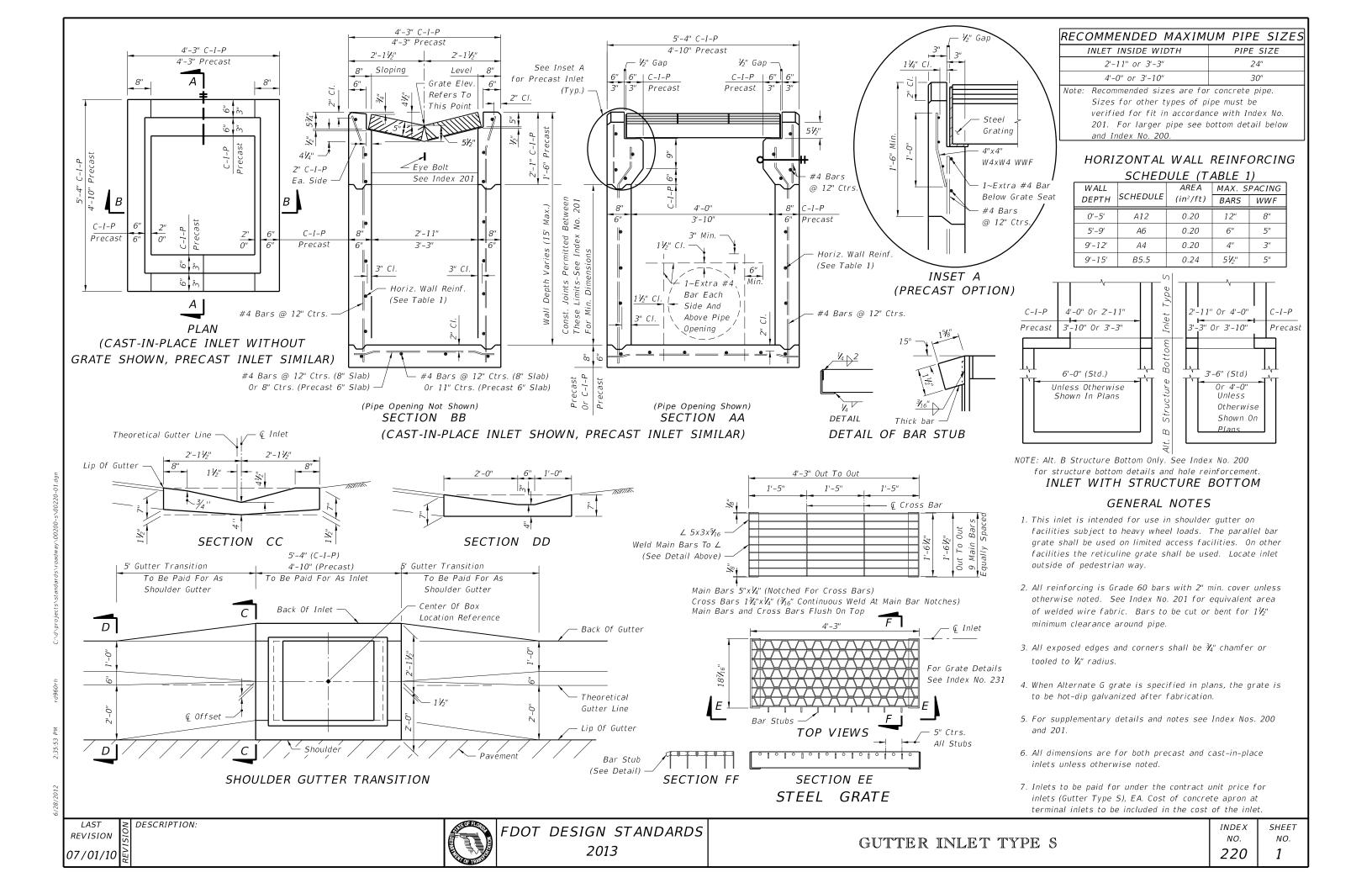


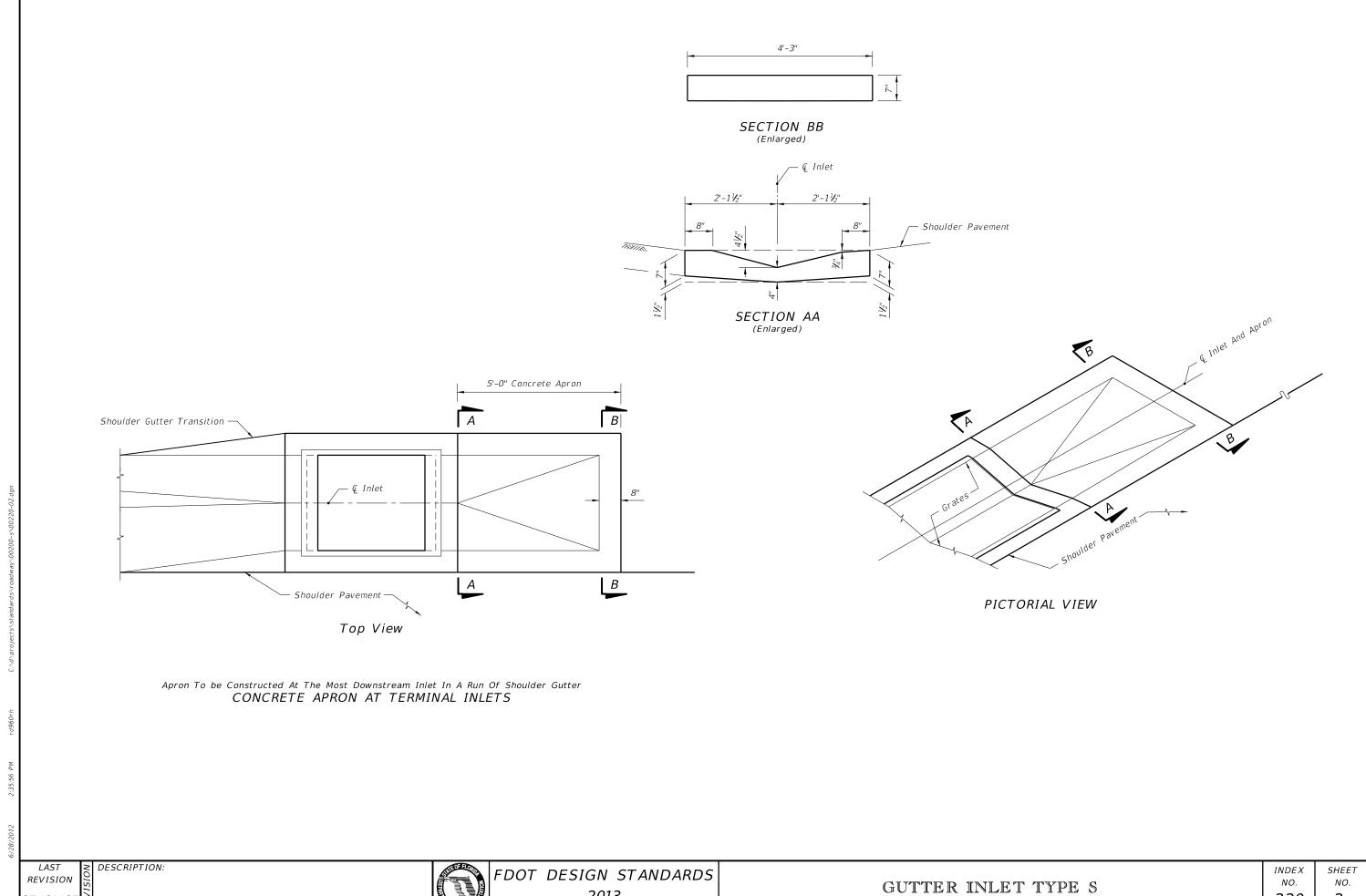
01/01/12

2'-6"

C-I-P

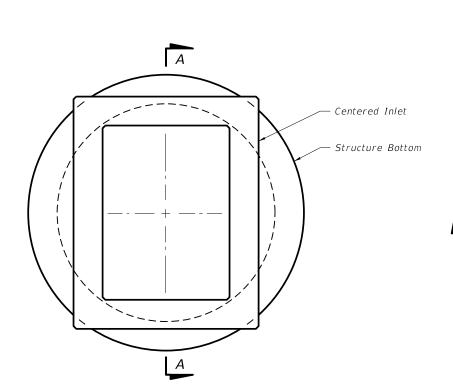






07/01/07





#4 Bar Each Corner (2-0" Min. Length)

2 Way Reinforcement See Tables

#5 Hoop Bar (Peripheral Reinforcement)

See Table For Dimensions

#8 Bars

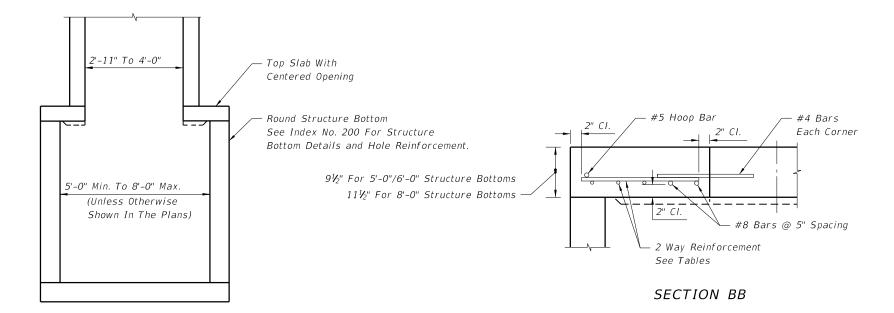
TOP SLAB REINFORCING DIAGRAM

TOP SLAB OPENINGS				
DIAMETER OPENING SIZE				
	MIN. MAX.			
5'-0" To 8'-0"	2'-11" x 4'-0"	3'-3" x 3'-10"		

SECTION AA

DESCRIPTION:

TOP VIEW



TOP SLAB WITH						
CEI	CENTERED OPENING					
SLAB DEPTH						
SIZE: 5'-0"						
≥0.5'<30'	91/2"	С				
30'-40'	9½"	D				
SIZE: 6'-0"						
≥0.5′<8′	91/2"	В				
8'<18'	9½"	С				
18'<30'	9½"	D				
30'<37'	9½"	Ε				
37'-40'	91⁄2"	G				
SIZE: 8'-0"						
≥0.5′<9′	11½"	С				
9'<15'	11½"	D				
15'<23'	11½"	Ε				
23'<33'	11½"	Ε				

111/2"

TOP SLAB

REINFORCING SCHEDULE

SCHEDULE

GRADE 60 (BAR)

OR 65 KSI & 70 KSI

(WIRE FABRIC) In²/ft.

0.20

0.53 0.73

ALT. A STRUCTURE BOTTOM FOR INLET TYPE S

LAST REVISION TO THE PROPERTY OF A SECOND TO THE PROPERTY OF T

TO HOLD

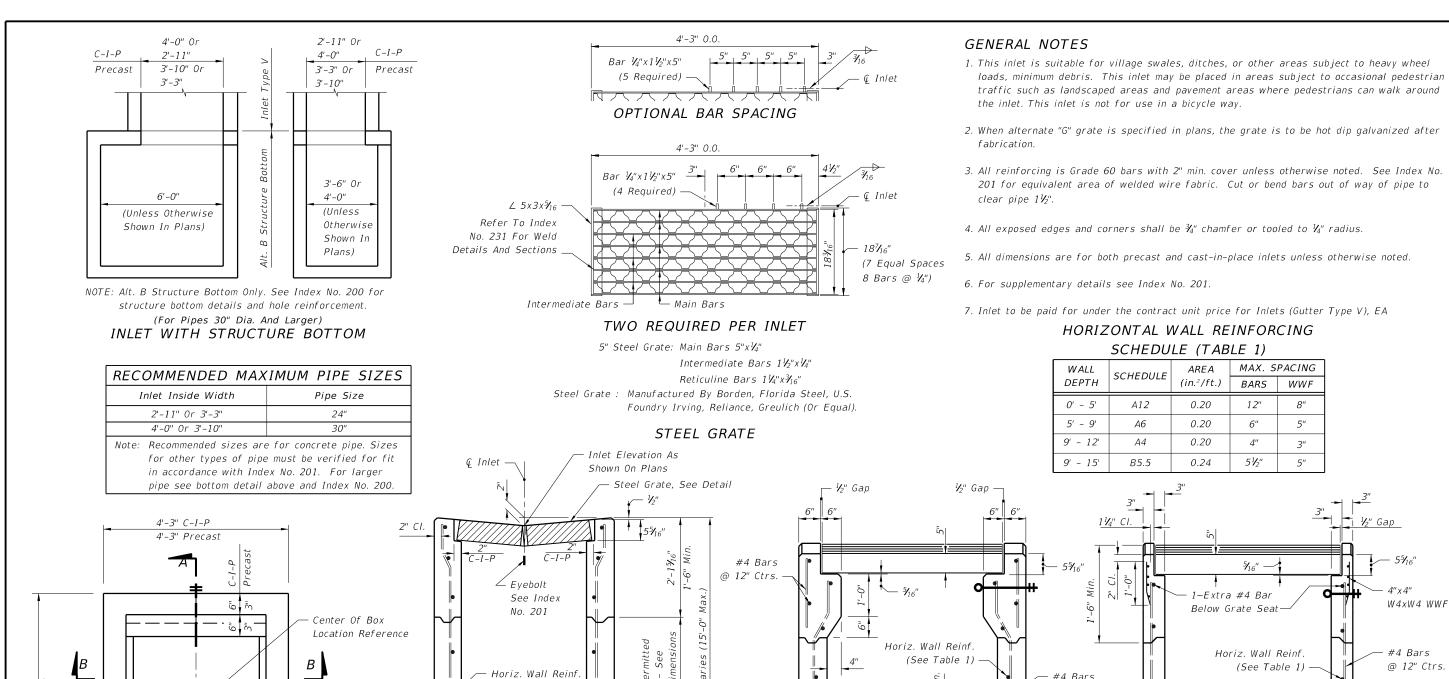
FDOT DESIGN STANDARDS
2013

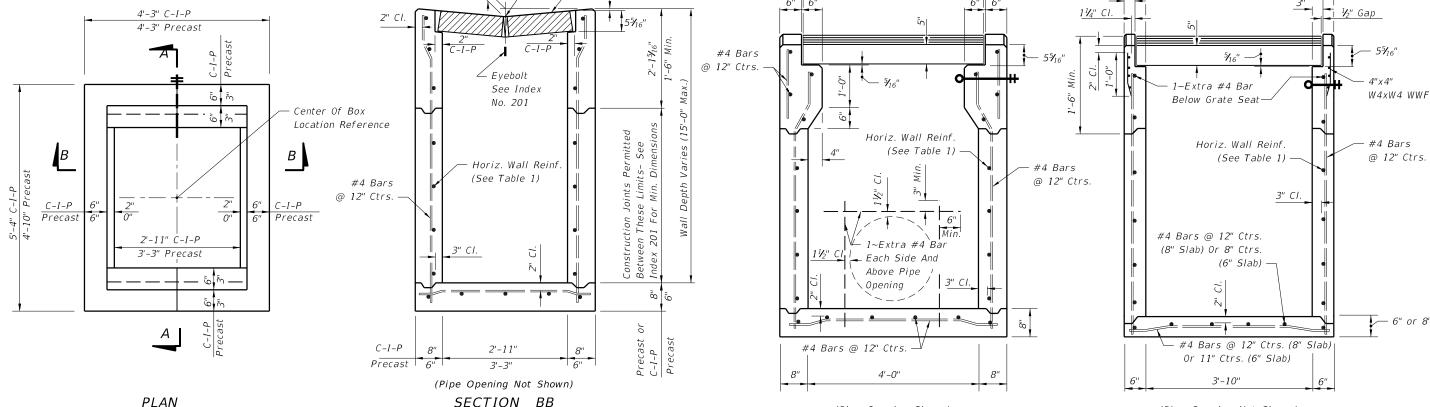
GUTTER INLET TYPE S

33'-40'

INDEX SHE NO. NO. NO. 3

SHEET NO. **3**





(CAST-IN-PLACE INLET SHOWN WITHOUT

GRATE; PRECAST INLET SIMILAR)



(CAST-IN-PLACE INLET SHOWN

PRECAST INLET SIMILAR)

(Pipe Opening Shown)

SECTION AA

(CAST-IN-PLACE INLET)

MAX. SPACING

WWF

5"

3"

5"

BARS

12"

6"

4"

5½"

(Pipe Opening Not Shown)

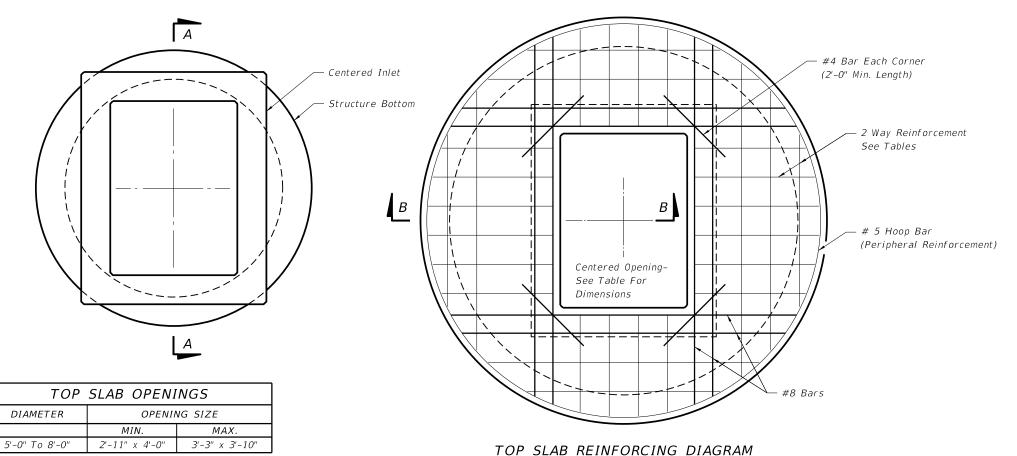
SECTION AA

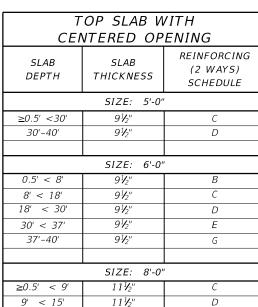
(PRECAST INLET)

0.20

0.20

0.24





111/2"

111/2"

111/2"

Ε

Ε

TOP SLAB

REINFORCING SCHEDULE

SCHEDULE

Α

В

15' < 23'

23' < 33'

33'-40'

GRADE 60 (BAR)

OR 65 KSI &

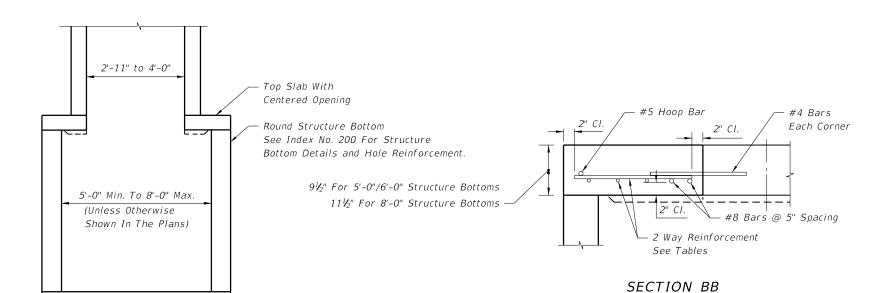
70 KSI (WIRE FABRIC)

In.²/ft.

0.20

0.24

0.37 0.53 0.73 1.06 1.45



ALT. A STRUCTURE BOTTOM FOR INLET TYPE V

LAST REVISION 0701/05

DESCRIPTION:

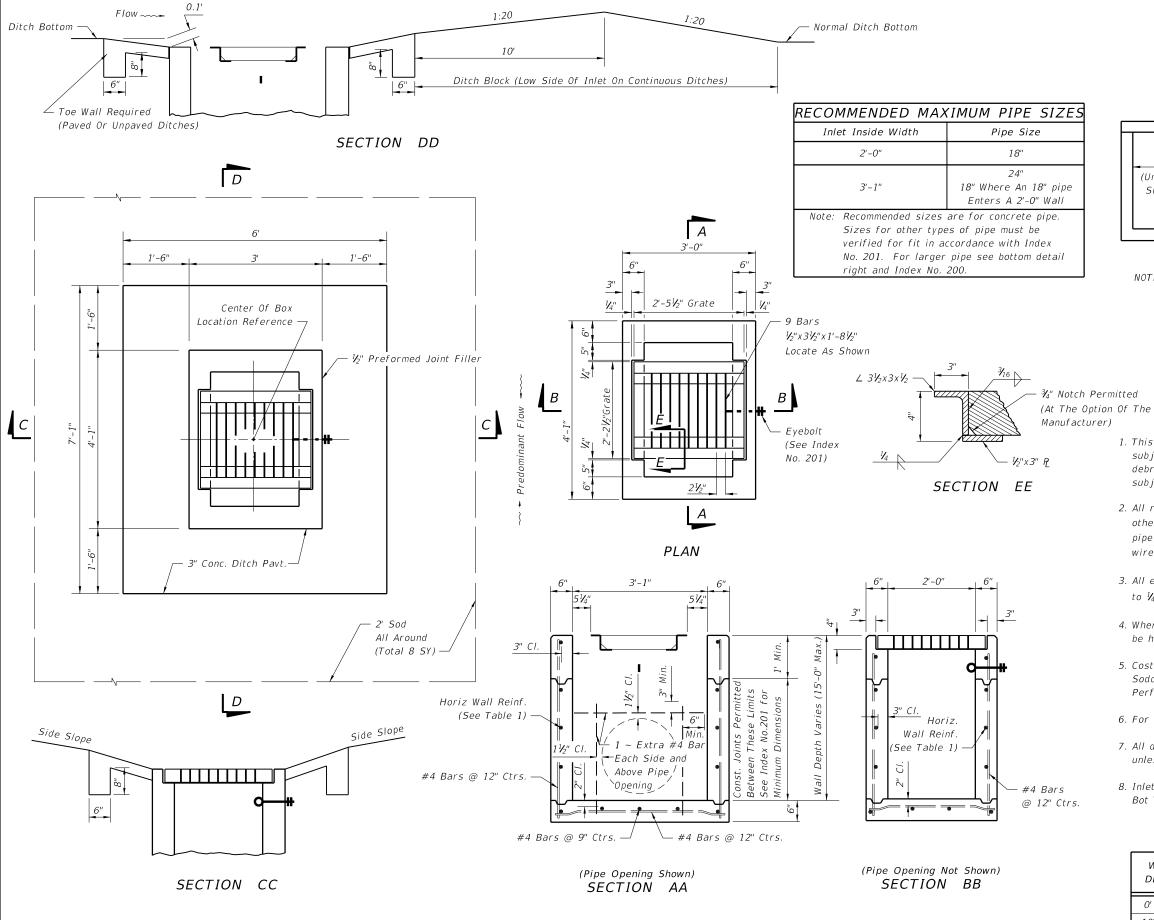


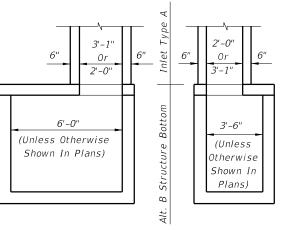
SECTION AA

FDOT DESIGN STANDARDS 2013

GUTTER INLET TYPE V

INDEX SHEET NO. NO. 221





NOTE: Alt. B Structure Bottom Only. See Index No. 200 for Structure Bottom Details And Hole Reinforcement.

INLET WITH STRUCTURE BOTTOM

GENERAL NOTES

- 1. This inlet is designed for ditches, medians, or other area subject to heavy wheel loads on limited access facilities where debris may be a problem. This inlet is not for use in areas subject to pedestrian and/or bicycle traffic.
- 2. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Cut or bend bars out of way of pipe to clear pipe by 1½". See Index 201 for equivalent area of welded wire fabric.
- 3. All exposed edges and corners shall be ¾" chamfer or tooled to 1/4" radius.
- 4. When alternate "G" grate is specified in plans, the grate is to be hot-dip galvanized after fabrication.
- 5. Cost of ditch paving to be included in the cost of Inlet. Sodding to be paid for under contract unit price for Performance Turf, SY.
- 6. For supplemental details see Index No. 201.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 8. Inlet to be paid for under the contract unit price for inlets (Dt Bot Type A), EA.

HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0' - 10'	A12	0.20	12"	8"
10' - 15'	A6	0.20	6"	5"

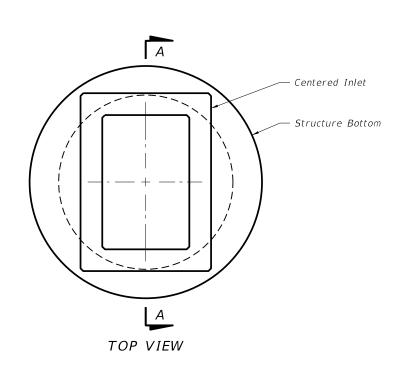
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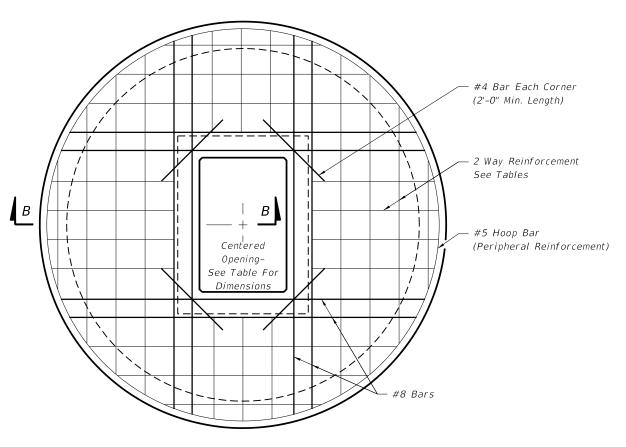


FDOT DESIGN STANDARDS 2013

DITCH BOTTOM INLET TYPE A

INDEX SHEET NO. NO. 230



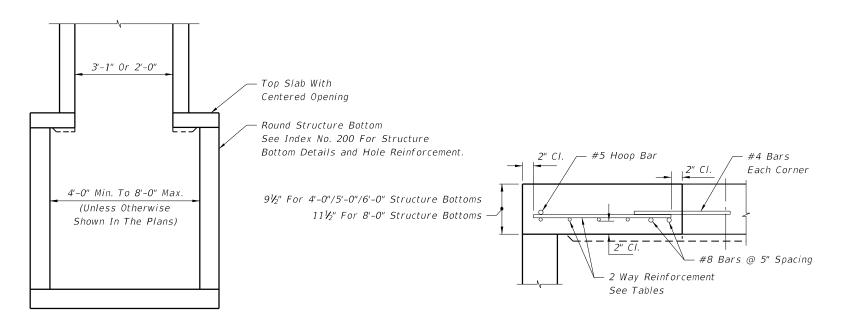


T	TOP SLAB			
REINFORCING SCHEDULE				
GRADE 60 (BAR) OR				
SCHEDULE	65 KSI & 70 KSI			
SCHEDULE	(WIRE FABRIC)			
	In.²/ft.			
Α	0.20			
В	0.24			
С	0.37			
D	0.53			
Ε	0.73			
F	1.06			
G	1.45			

TOP SLAB	OPENINGS	
DIAMETER	OPENING SIZE	
	MIN.	
4'-0" To 8'-0"	2'-0" x 3'-1"	

TOP SLAB REINFORCING DIAGRAM

SECTION BB



TOP SLAB WITH						
CENTERED OPENING						
SLAB DEPTH	REINFORCING (2 WAYS) SCHEDULE					
	SIZE: 4'-0'					
≥0.5'-40'	≥0.5'-40' 9½" C					
	SIZE: 5'-0"					
≥0.5'<30' 9½" C						
30'-40' 9½" D						
SIZE: 6'-0"						
0.5'<8'	9 ½ "	В				
0.5'<8' 8'<18'	9½" 9½"	B C				
	_					
8'<18'	9½"	С				
8'<18' 18'<30'	9½" 9½"	C D				
8'<18' 18'<30' 30'<37'	9½" 9½" 9½"	C D E				
8'<18' 18'<30' 30'<37'	9½" 9½" 9½"	C D E G				
8'<18' 18'<30' 30'<37'	9½" 9½" 9½" 9½"	C D E G				
8' < 18' 18' < 30' 30' < 37' 37' - 40'	9½" 9½" 9½" 9½" 9½"	C D E G				
8'<18' 18'<30' 30'<37' 37'-40' ≥0.5'<9'	9½" 9½" 9½" 9½" 9½" SIZE: 8'-0'	C D E G				
8'<18' 18'<30' 30'<37' 37'-40' ≥0.5'<9' 9'<15'	9½" 9½" 9½" 9½" 9½" 5IZE: 8'-0' 11½"	C D C C D C C D D				

ALT. A STRUCTURE BOTTOM FOR INLET TYPE A

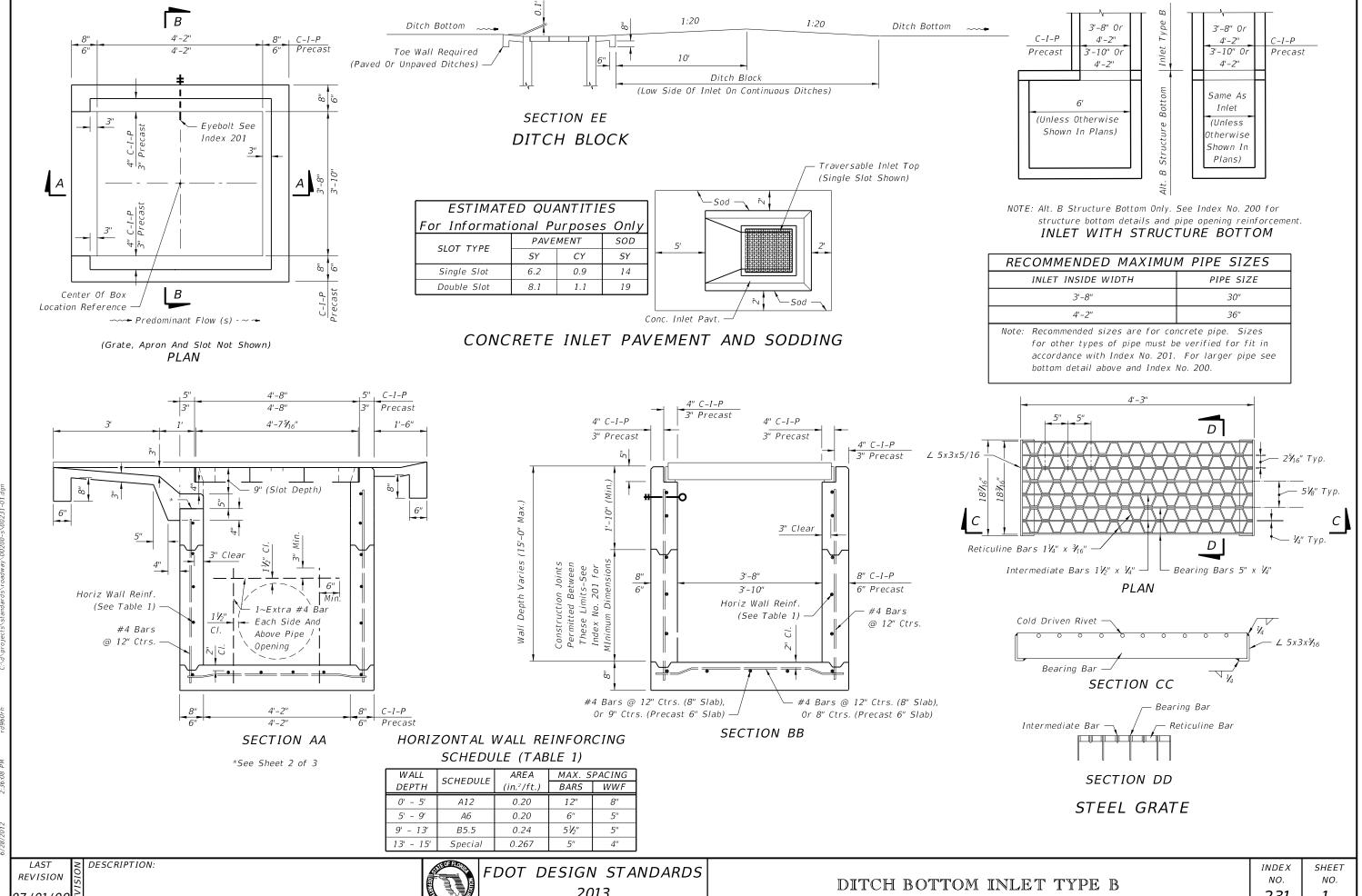
LAST REVISION

FDOT DESIGN STANDARDS
2013

DITCH BOTTOM INLET TYPE A

INDEX SHEET NO. NO. 230 2

SECTION AA

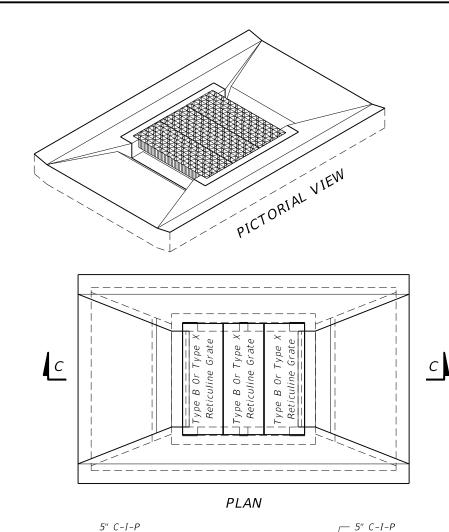


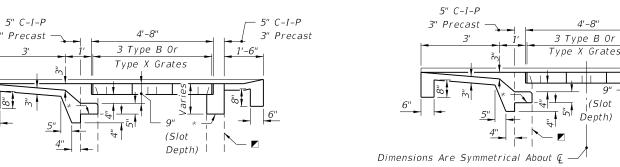
07/01/09



A







PICTORIAL VIEW

В

PLAN

SECTION AA

SINGLE SLOT

4" C-I-P or 3" Precast

SECTION BB

Inlet Box (Line Type Indicates Existing Box To Facilitate Depiction Of Partial Construction On Existing Inlets)

SECTION CC DOUBLE SLOT

* On new boxes the traversable top may be cast as a monolithic unit or cast in segments, and the location of this line may be lower to facilitate handling and placement; however, the slot depth is to remain at 9 inches. See Index No. 201 for top to wall connection. For converting to traversable tops on existing inlets remove concrete to this line and expose the existing reinforcement. Reshape or splice in reinforcement to penetrate the rim and returns of the grate seat, and bend the reinforcement into the slot shelf to extend into the abutting throat pavement.

TRAVERSABLE TOPS FOR INLETS TYPE B AND FOR CONVERSIONS OF EXISTING INLETS TYPE B AND TYPE X

GENERAL NOTES

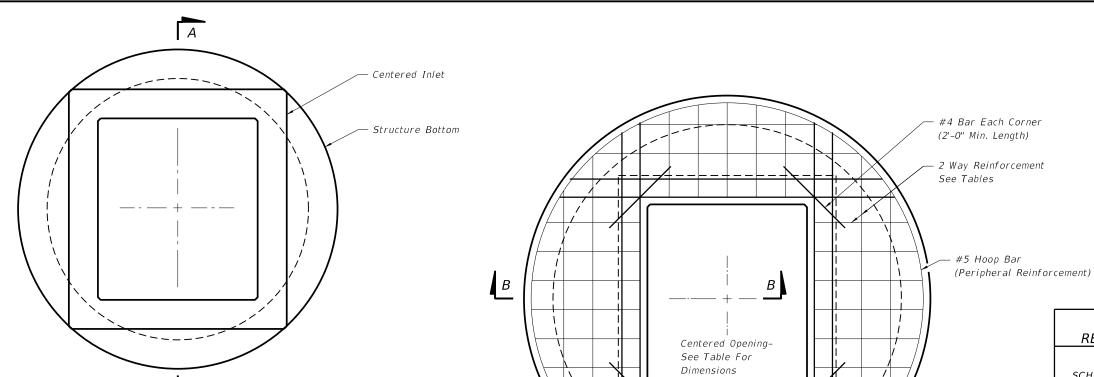
- 1. The general purpose of the inlet top designs are:
 - a. For ditches, medians or other areas subject to heavy wheel loads. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet. Inlet not suitable for bicycle traffic.
 - b. Provide full grate and horizontal slot designs for new construction.
 - c. Provide full grate and horizontal slot designs for replacing the vertical slot tops on existing Inlets Type B and Type X that are in locations subject to occasional pedestrian traffic.
- 2. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. See Index No. 201 for equivalent area of welded wire fabric. Bars to be cut or bent for min. 11/2" clearance around pipe.
- 3. All exposed edges and corners shall be $\frac{3}{4}$ " chamfer or tooled to $\frac{1}{4}$ " radius.
- 4. When Alternate G grates are specified in the plans, the grates are to be hot-dip galvanized after fabrication.
- 5. Cost for constructing traversable tops on new inlet boxes shall be included in the contract unit price for Inlets (DT BOT) (Type B), EA., and shall include the cost for surrounding concrete inlet pavement. Existing Inlets Type B and Inlets Type X that are converted to traversable inlet tops shall be paid for under the contract unit price for Inlets (DT BOT) (Type B) (Partial), EA. Unit price and payment shall be full compensation for inlet conversion and shall include the removal and disposal of any existing concrete inlet pavement; the removal and stockpiling or disposal of sufficient material from the existing inlet box to facilitate construction of the required inlet top; construction of the required inlet conversion; backfill construction; construction of concrete inlet pavement; reusing, supplementing, transferring or replacing grates as required by plans or as directed by the Engineer; any required earthwork for ditch restoration within 30' of the inlet; and, restoration of disturbed turf.
- 6. Ditch pavement shall be paid for, separate from the inlet and concrete inlet pavement, by pavement types and units as called for in the plans.
- 7. Sod will be paid for under the contract unit price for Performance Turf, SY.
- 8. For supplementary details see Index No. 201.
- 9. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.

DESIGN NOTES

- 1. The type of top (single or double slots) depends on the approach ditch configuration and the hydraulic requirements of the site. The designer will stipulate in the plans the type of top to be constructed at each individual inlet location.
- 2. On existing inlets, conversion grates shall be constructed at the original grate elevations unless other elevations are called for in the plans. When plans call for the inlet top to be constructed to support storm water detention, details for ditch modifications and underdrains shall be shown in the plans.

MAINTENANCE NOTES

1. Traversable inlet tops that are constructed by maintenance contract or by maintenance forces may reuse the existing grates that are determined by the Maintenance Engineer to be functionally sound, and their reuse is so directed by the Maintenance Engineer. Existing grates approved for reuse and new grates may be mixed, matched or replaced as directed by the Maintenance Engineer.



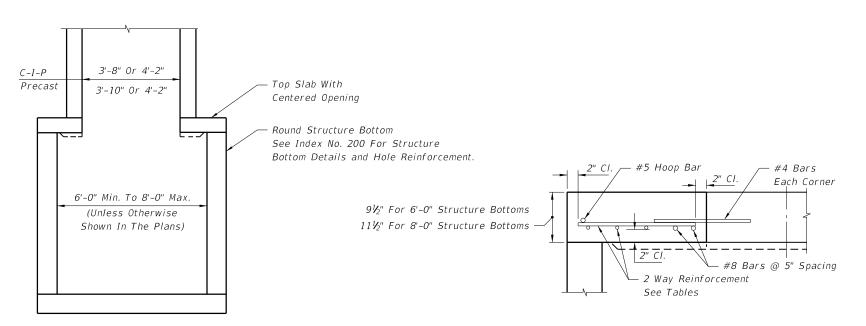
TOP SLAB OPENINGS			
	DIAMETER	OPENIN	IG SIZE
		MIN.	MAX.
	6'-0" to 8'-0"	3'-8" x 4'-2"	3'-10" x 4'-2"

SECTION AA



	TOP	SLAB	REINFORCING	DIAGRAM
--	-----	------	-------------	---------

SECTION BB



TOP SLAB WITH					
CEN	CENTERED OPENING				
SLAB SLAB REINFORCI DEPTH THICKNESS SCHEDUL					
	SIZE: 6'-0"				
0.5' < 8'	91⁄2"	В			
8' < 18"	9⅓"	С			
18' < 30'	9⅓"	D			
30' < 37'	91⁄2"	E			
37'-40'	91⁄2"	G			
SIZE: 8'-0"					
≥0.5′ < 9′	11½"	С			
9' < 15'	11½"	D			
15' < 23'	11½"	E			
23' < 33'	11½"	Е			
33'-40'	11½"	G			

ALT. A STRUCTURE BOTTOM FOR INLET TYPE B

LAST REVISION 07/01/05

DESCRIPTION:

FDOT DESIGN STANDARDS 2013

DITCH BOTTOM INLET TYPE B

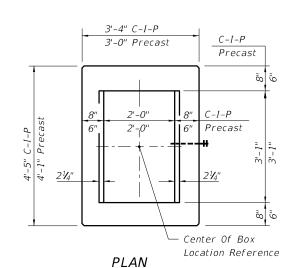
#8 Bars

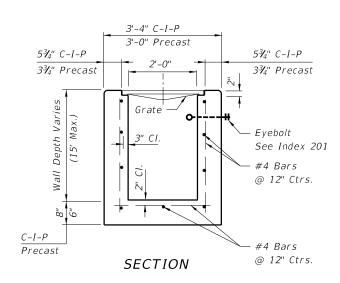
SHEET INDEXNO. NO. 231 3

LAST

07/01/05







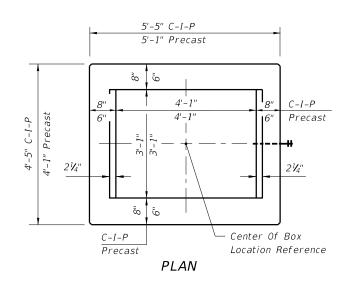
HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 1)

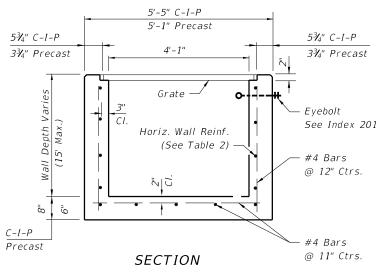
WALL	CCHEDITIE	SCHEDULE AREA		. SPACING	
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF	
0'-15'	A12	0.20	1 <i>2</i> "	8"	

TYPEC

Recommended Maximum Pipe Size:

2'-0" Wall - 18" Pipe 3'-1" Wall - 24" Pipe (18" where an 18" pipe enters a 2'-0" wall)





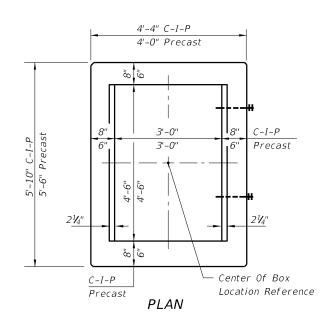
HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 2)

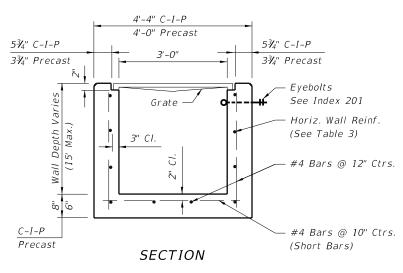
	WALL	COUEDIUE AREA		MAX. SPACING	
	DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
ĺ	0'-6'	A12	0.20	12"	8"
	6'-10'	A6	0.20	6"	5"
	10'-13'	Α4	0.20	4"	3"
	10'-15'	B5.5	0.24	5½"	5"

TYPE D

Recommended Maximum Pipe Size:

3'-1" Wall - 24" Pipe 4'-1" Wall - 36" Pipe





HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 3)

WALL	SCHEDULE	AREA	MAX. SPACING							
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF						
0'-5'	A12	0.20	12"	8"						
0'-7.5'	A6	0.20	6"	5"						
7.5'-10'	B5.5	0.24	5½"	5"						
10'-15'	C6.5	0.37	6½"	6"						

TYPE E

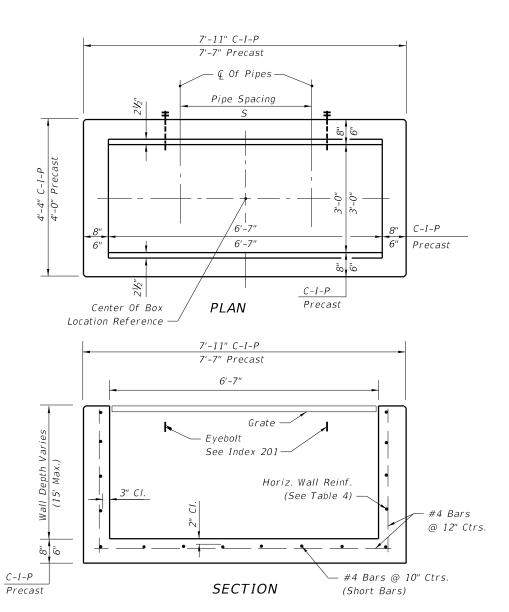
Recommended Maximum Pipe Size:

3'-0" Wall - 24" Pipe 4'-6" Wall - 36" Pipe

SHEET

NO.

DESCRIPTION:

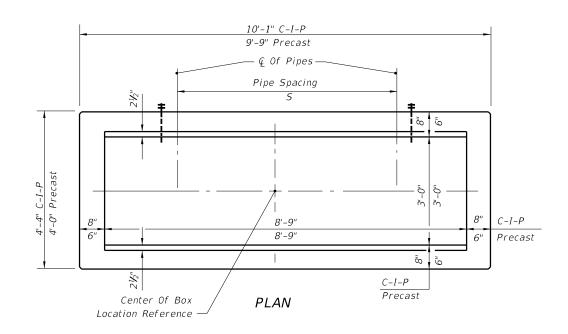


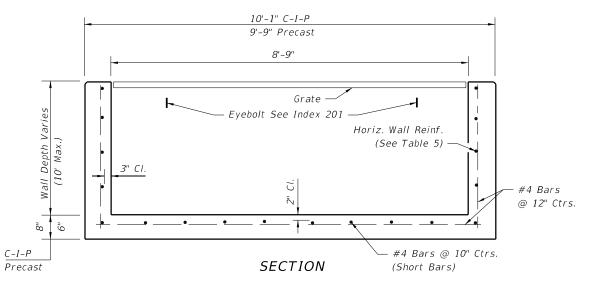
HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 4)

WALL	SCHEDULE	AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0'-5'	B5.5	0.24	5½"	5"
5'-7'	C6.5	0.37	6½"	6"
7'-15'	D4.5	0.53	4½"	4"

TYPE H (2 & 3-GRATE INLET)

Recommended Maximum Pipe Size: 3'-0" Wall - 24" Pipe 6'-7" Wall - 1-60" Pipe Or 2-24" Pipe (S=3'-5")





HORIZONTAL WALL REINFORCING SCHEDULES (TABLE 5)

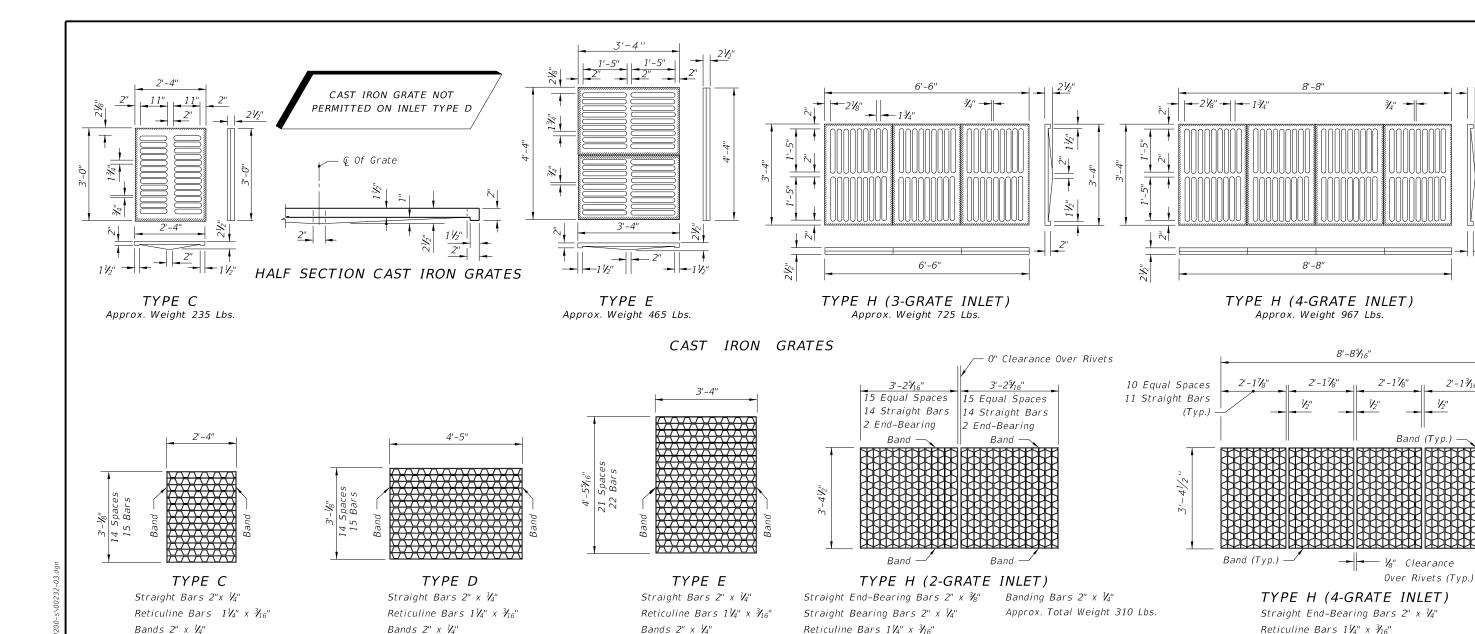
WALL		AREA	MAX. S	PACING
DEPTH	SCHEDULE	(in.²/ft.)	BARS	WWF
0'-5'	C3.5	0.37	31⁄2"	3"
5'-10'	D4.5	0.53	4½"	4"

TYPE H (4-GRATE INLET)

Recommended Maximum Pipe Size: 3'-0" Wall - 24" Pipe 8'-9" Wall - 1-78" Pipe Or 2-30" Pipe (S=4'-3")

GENERAL NOTES See Sheet 3 of 7.





Approx. Weight 215 Lbs.

STEEL GRATES

NOTE: Steel Grates Are Required On Inlets With Traversable Slots And On Inlets where Bicycle Traffic Is Anticipated.

GENERAL NOTES

1. These inlets are suitable for bicycle traffic and are to be used in ditches, medians and other areas subject to infrequent traffic loadings but are not to be placed in areas subject to any heavy wheel loads. These inlets may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet.

Approx. Weight 190 Lbs.

- 2. Inlets subject to minimal debris should be constructed without slots. Where debris is a problem inlets should be constructed with slots. Slotted inlets located within roadway clear zones and areas subject to pedestrians shall have traversable slots. The traversable slot modification is not adaptable to inlet Type H. Slots may be constructed at either or both ends as shown on plans. Traversable slots shall not be used in areas subject to occasional bicycle traffic.
- 3. Steel grates are to be used on all inlets where bicycle traffic is anticipated. Steel grates are to be used on all inlets with traversable slots. Either cast iron or steel grates may be used on inlets without slots where bicycle traffic is not anticipated. Either cast iron or steel grates may be used on all inlets with

- non-traversable slots. Subject to the selection described above, when Alternate G grate is specified in the plans, either the steel grate, hot dip galvanized after fabrication, or the cast iron grate may be used, unless the plans stipulate the particular type.
- 4. Recommended maximum pipe sizes shown are for concrete pipe. Size for other types of pipe must be checked for fit.
- 5. All exposed edges and corners shall be $\frac{3}{4}$ " chamfer or tooled to $\frac{1}{4}$ " radius.
- 6. Concrete inlet pavement to be used on inlets without slots and inlets with non-traversable slots only when called for in the plans; but required on all traversable slot inlets. Cost to be included in contract unit price for inlets. Quantities shown are for information only.
- 7. Traversable slots constructed in existing inlets shall be paid for as inlets partial. For conversion work and method of payment see 'TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS'

8. Sodding to be used on all inlets not located in paved areas and paid for under contract unit price for Performance Turf, SY.

Banding Bars 2" x ¾₁₆" Approx. Total Weight 388 Lbs.

- 9. For supplementary details see Index No. 201.
- 10. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Bars to be cut or bent for 1½" clearance around pipe opening. Provide one additional #4 bar above and at each side of pipe opening.

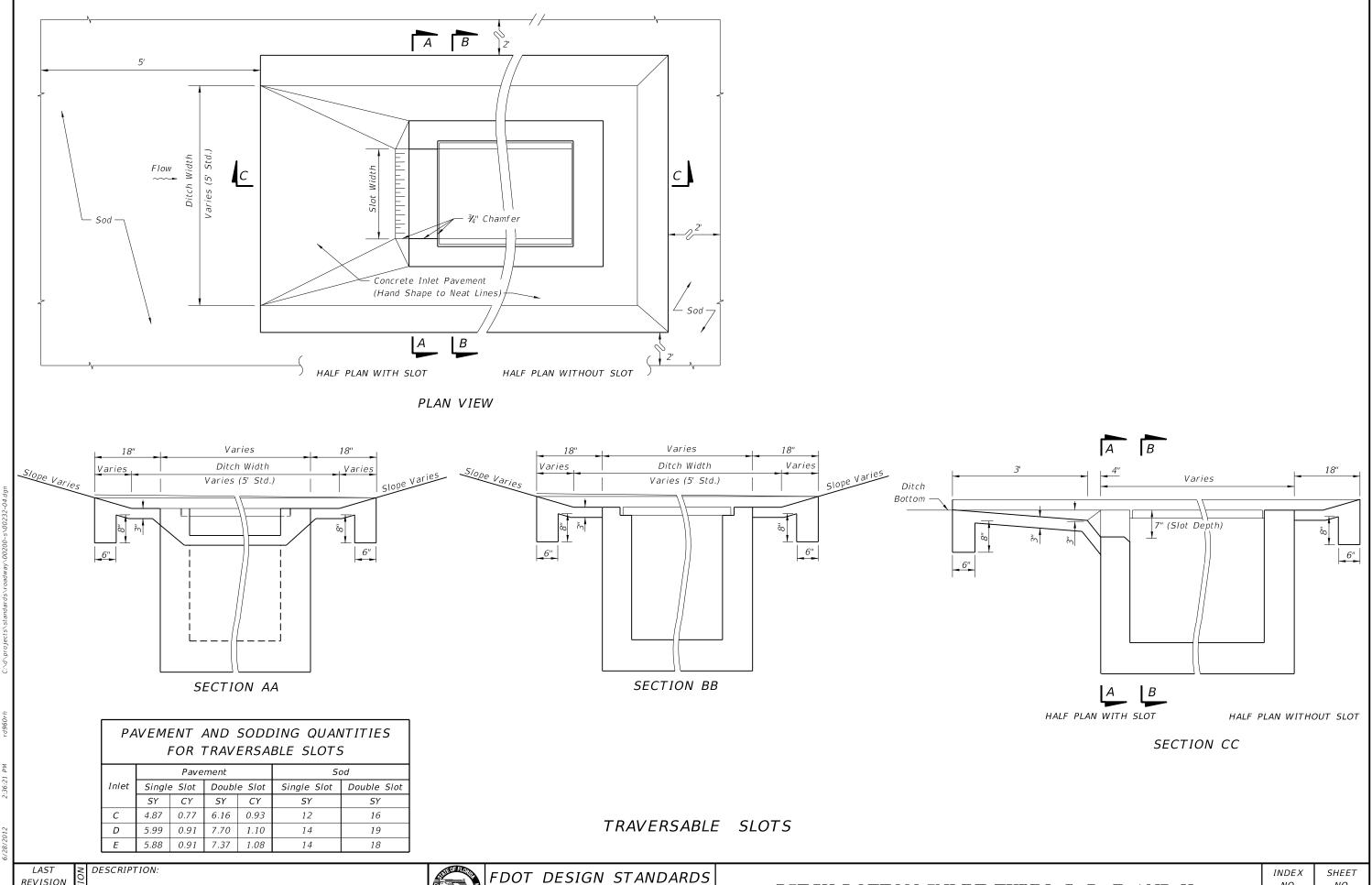
DESCRIPTION: LAST REVISION 07/01/10

Approx. Weight 104 Lbs.

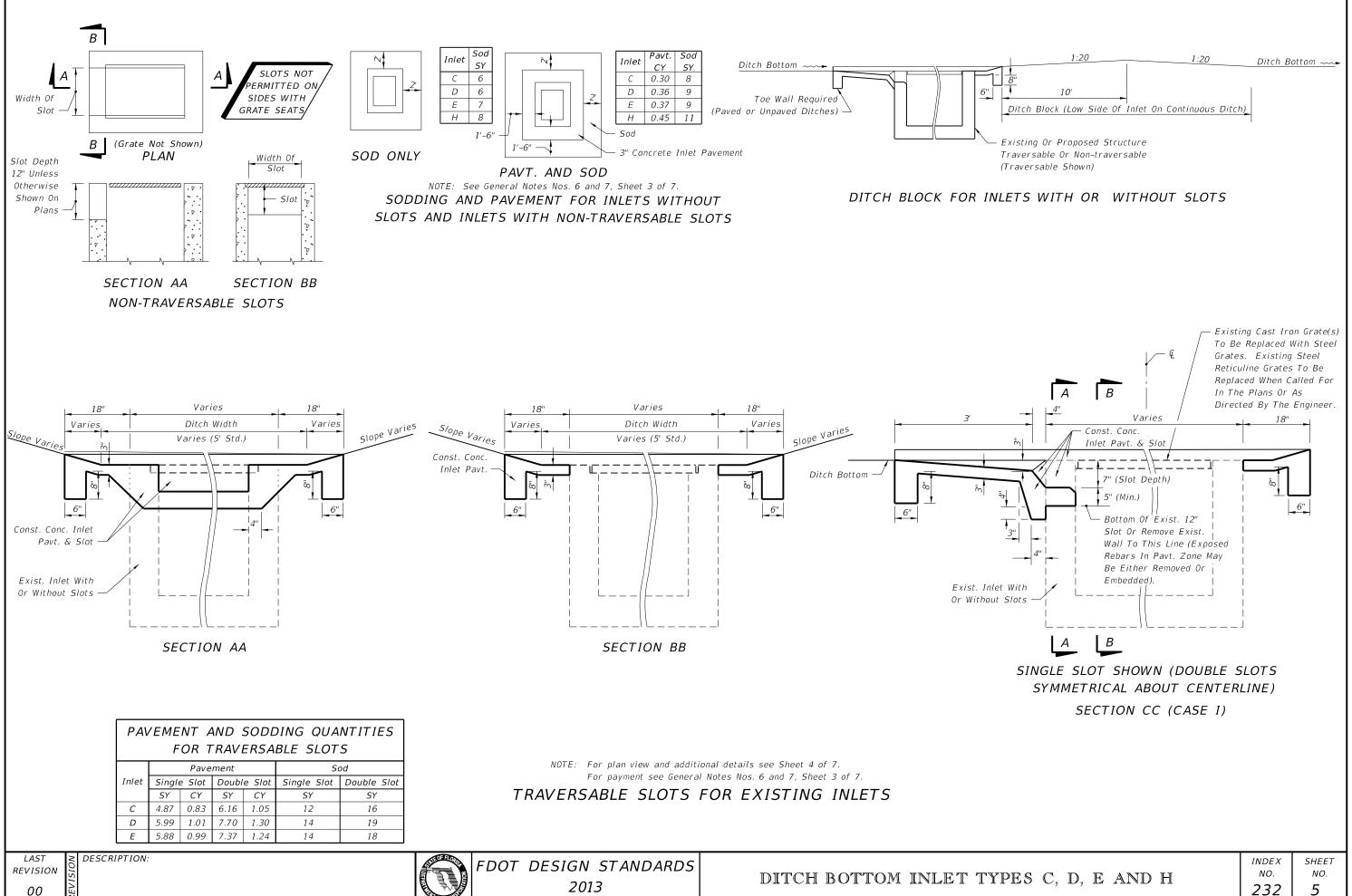


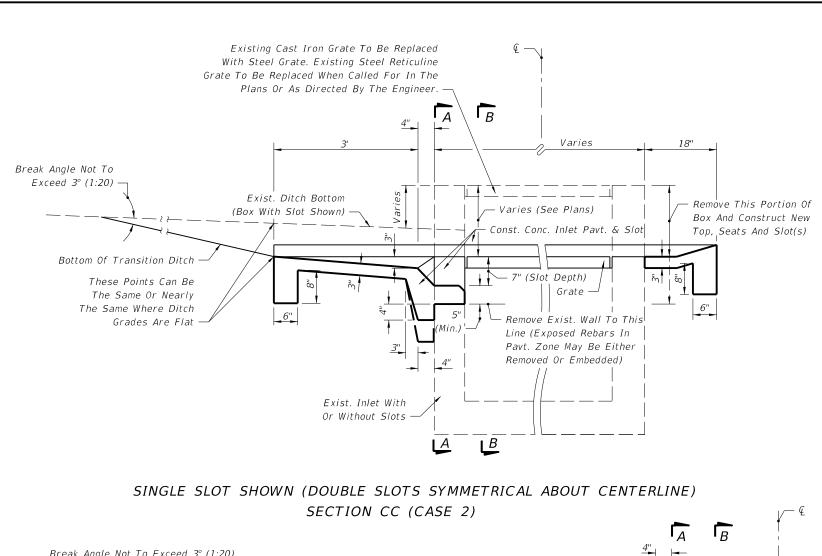
FDOT DESIGN STANDARDS 2013

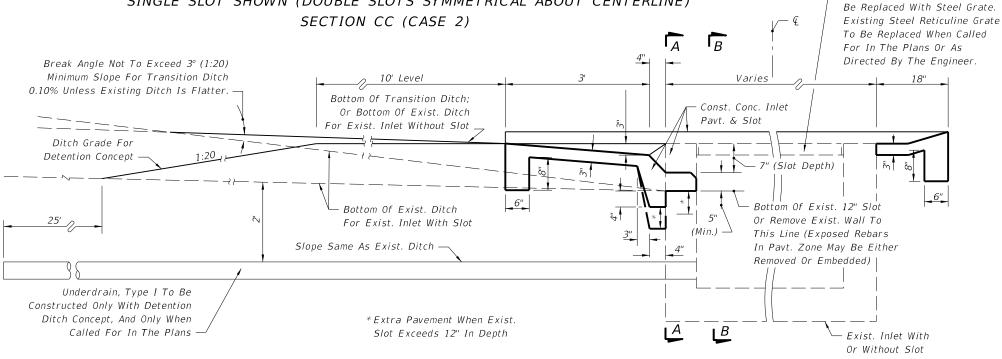
Clearance



REVISION 07/01/07







SINGLE SLOT SHOWN (DOUBLE SLOTS SYMMETRICAL ABOUT CENTERLINE) SECTION CC (CASE 3)

TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

DESIGN NOTES FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- 1. The general purpose of these conversions is to remove the hazard of the protruding inlet top, while not creating a hazard by depressing the top too deeply.
- 2. The corrective procedure depends on the approach ditch grade and hydraulic requirements of the site. The selection of the appropriate case depends on the relationship between inlet top and ditch elevation, and, on the vertical clearance between the top of the uppermost pipe(s) and the grate. The purpose for the Case 1 conversion is to add the traversable slot to an existing inlet where top removal, change in grate elevation and ditch transitions are not required. Case 2 will normally be applicable to ditches with flatter grades adjoining the inlet. Case 3 will normally be applicable to ditches with steeper grades adjoining the inlet where build up of the existing ditch is acceptable.
- 3. The designer shall stipulate in the plans which case is to be constructed at each individual inlet location.

Where the existing inlet top is above the existing ditch (Case 2) but borrow material will be required to adjust the ditch (Case 3), and vertical clearance or other conditions do not prevent removal of the inlet top, the designer should call for Case 2. The designer shall determine if ditch reconstruction is required more than 35 feet beyond any traversable slot side and shall include separate pay items in the plans to cover the cost for that portion of required ditch reconstruction exceeding the 35 foot limit. The designer shall also determine whether ditch pavement is required for ditch restoration within the 35 foot limit and include that pavement under a pay item separate from the inlets partial.

When the detention ditch concept is to be used with Case 3, the designer shall stipulate 'Case 3 (Detention)' in the plans.

The designer shall determine whether tight soil or other conditions at each individual inlet indicates the need for underdrain in Case 3 conversions and shall call for Underdrain, Type I in the plans.

METHOD OF PAYMENT FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- 1. Existing inlets converted to traversable slot tops under Cases 1, 2 and 3 shall be paid for as inlets partial, each. Case shall not be included in the pay item
- 2. All ditch reconstruction work within 35 feet of each traversable slot conversion, whether required by these details or as a direct result of the conversion, shall be included as a part of the partial cost. Reconstruction work shall include excavation and removal of surplus materials or borrow materials in place, grading, compaction, shaping and restoration of disturbed turf. Sodding, ditch pavement and underdrain are not included as part of the inlet partial cost and are to be paid for separately.
- 3. Concrete inlet pavement and sodding shall be in accordance with the sections on this detail and with the Plan on Sheet 4 and Sections AA, BB and CC (as Case 1) and tabular quantities on Sheet 5.
- 4. Unit price and payment shall constitute full compensation for inlet conversion (including concrete inlet paving and replacement grate(s)), ditch reconstruction, restoration of disturbed turf, and shall be paid for under the contract price for Inlets (DT Bot) (Type __) (Partial), each.

Sodding shall be paid for under the contract unit price for Performance Turf, SY.

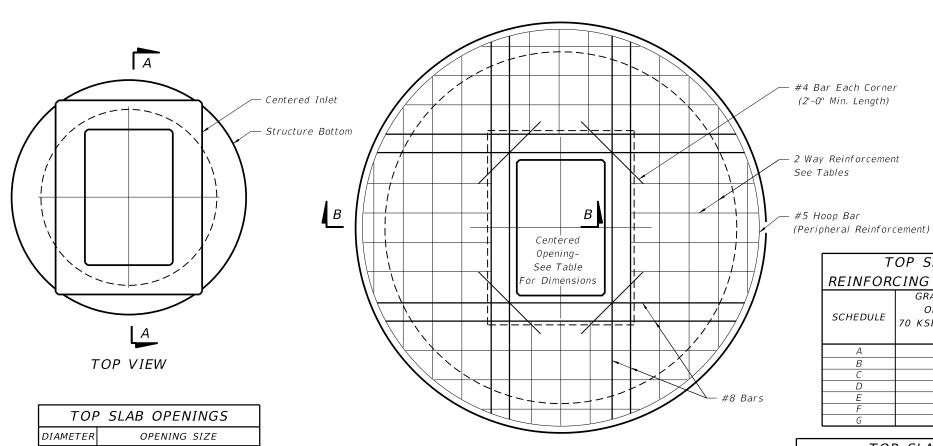
Ditch pavement shall be paid for separate from the inlet by pavement type(s) and unit(s) as called for in the plans.

LAST REVISION 07/01/07

DESCRIPTION:



Existing Cast Iron Grate To



Type C 3'-1" Type C 2'-0" Type D 4'-1" Type D 3'-1" Type E 4'-6" Type E 3'-0" Struct 3'-6" Unless Otherwise Unless Otherwise Shown On Plans Shown On Plans В

See Index No. 200 for structure bottom details and hole reinforcement.

ALT. B STRUCTURE BOTTOM FOR INLETS TYPE C, D & E

T	OP SLAB
REINFOR	CING SCHEDULE
	GRADE 60 (BAR)
COUEDINE	OR 65 KSI &
SCHEDULE	70 KSI (WIRE FABRIC)
	In.²/ft.
Α	0.20
В	0.24
С	0.37
D	0.53
Ε	0.73
F	1.06
G	1.45

TOF	SLAB OPI	ENINGS
DIAMETER	OPENII	NG SIZE
	MIN.	MAX.
4'-0"	2'-0" x 3'-1"	2'-0" x 3'-1"
5'-0"	2'-0" x 3'-1"	3'-1" x 4'-1"
6'-0"	2'-0" x 3'-1"	3'-0" x 4'-6"
8'-0"	2'-0" x 3'-1"	3'-0" x 4'-6"

Type C 3'-1" Type D 4'-1" Type E 4'-6"

TYPE C 4'-0"

TYPE D 6'-0"

TYPE E 6'-0"

(Minimum Diameter Unless Otherwise Shown In The Plans) Top Slab With Centered Opening

— Round Structure Bottom

See Index No. 200 For

Structure Bottom Details

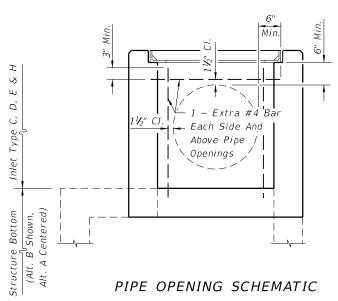
and Hole Reinforcement.

TOP SLAB REINFORCING DIAGRAM

 $\sim 9\frac{1}{2}$ " For 4'-0"/5'-0"/6'-0" Structure Bottoms 11½" For 8'-0" Structure Bottoms

7	OP SLAB W	/ITH
CEI	NTERED OP	ENING
SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE
	SIZE: 4'-0"	
≥0.5'-40'	91/2"	С
	SIZE: 5'-0"	
≥0.5′ < 30′	9½"	C
30'-40'	9½"	D
	SIZE: 6'-0"	
0.5' < 8'	9½"	В
8' < 18'	91/2"	С
18' < 30'	91/2"	D
30' < 37'	91/2"	Е
37'-40'	9½"	G
	SIZE: 8'-0"	
≥0.5′ < 9′	111/2"	С
9' < 15'	111/2"	D
15' < 23'	111/2"	E
23' < 33'	111/2"	E
33'-40'	11½"	G





SECTION AA

ALT. A STRUCTURE BOTTOM FOR INLETS TYPE C, D AND E

#4 Bars

#8 Bars @ 5" Spacing

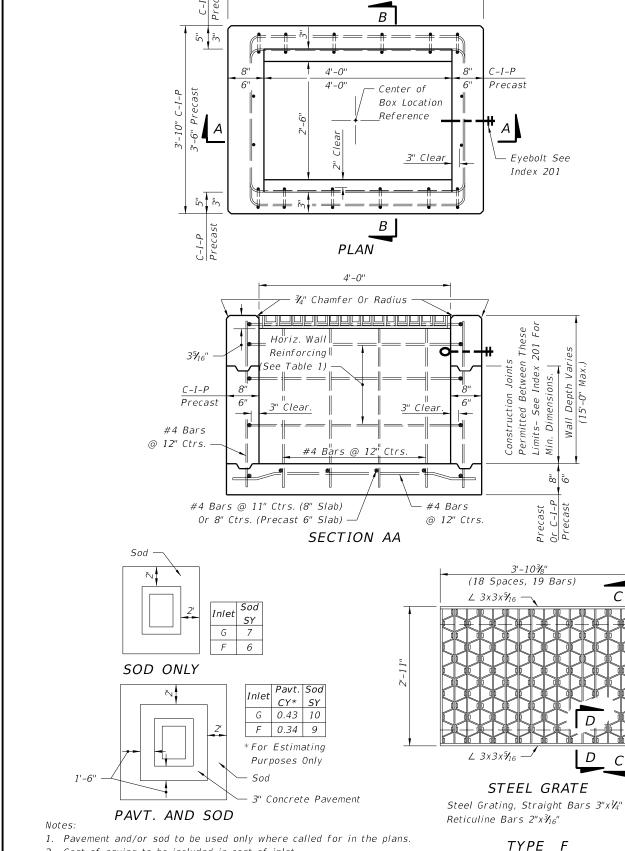
LAST REVISION 07/01/05

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2 Way Reinforcement

See Tables

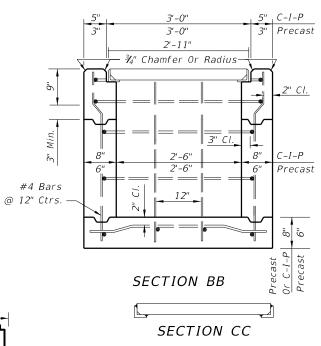
SECTION BB

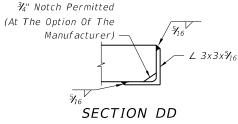


5'-4" C-I-P 5'-0" Precast

~ Extra #4 1½"CI. Bar Each Side And Above Pipe Openings Masonry Seal For Precast Inlets See Index 201

(TYPE F SHOWN, TYPE G SIMILAR) PIPE OPENING SCHEMATIC





HORIZONTAL WALL REINF. SCHEDULES TYPE F INLET (TABLE 1)

WALL	SCHEDULE	AREA	MAX. SPACING								
DEPTH	SCHEDOLE	(in²/ft)	BARS	WWF							
0' - 4'	A12	0.20	12"	8"							
4' - 7'	A6	0.20	6"	5"							
7' - 12'	B5.5	0.24	5 ½ ″	5"							
12' - 15'	Special 1	0.267	5"	4"							

GENERAL NOTES

- 1. These inlets are designed for use in ditches, medians, pavement areas, or other areas subject to heavy wheel loads, minimal debris, and bicycle traffic. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet. When inlet is placed in areas subject to bicycle traffic, install filler bar when clearance or gap is greater than 1" as shown in Index 218 Inset B.
- 2. When Alternate G grate is specified in plans, the grate is to be hot dip galvanized after fabrication.
- 3. These inlets may be used with Alternate B structure bottoms, Index 200. The inlet and bottom combinations are to be paid for under the contract unit price for inlets (DT Bot) (Type F (or G)) (J Bot, Depth), Ea.
- 4. All exposed edges and corners shall be $\frac{y}{4}$ chamfer or tooled to $\frac{y}{4}$ radius.
- 5. For supplemental details, see Index 201.
- 6. All reinforcing is Grade 60 bars with 2" min. cover unless otherwise noted. Bars to be cut or bent for 1½" clearance around pipe opening. Provide one additional #4 bar above and at each side of pipe opening, as shown.
- 7. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.

RECOMMENDED MAXI	MUM PIPE SIZES
INLET INSIDE WIDTH	PIPE SIZE
2'-6" (Type F)	18"
4'-0" (Type F)	30"
4'-10" / 5'-0" (Type G)	42"
·	•

Note: Recommended sizes are for concrete pipe. Sizes for other types of pipe must be verified for fit in accordance with Index No. 201. For larger pipe sizes see Note 3.

PAVEMENT AND SODDING

DESCRIPTION: LAST REVISION 01/01/10

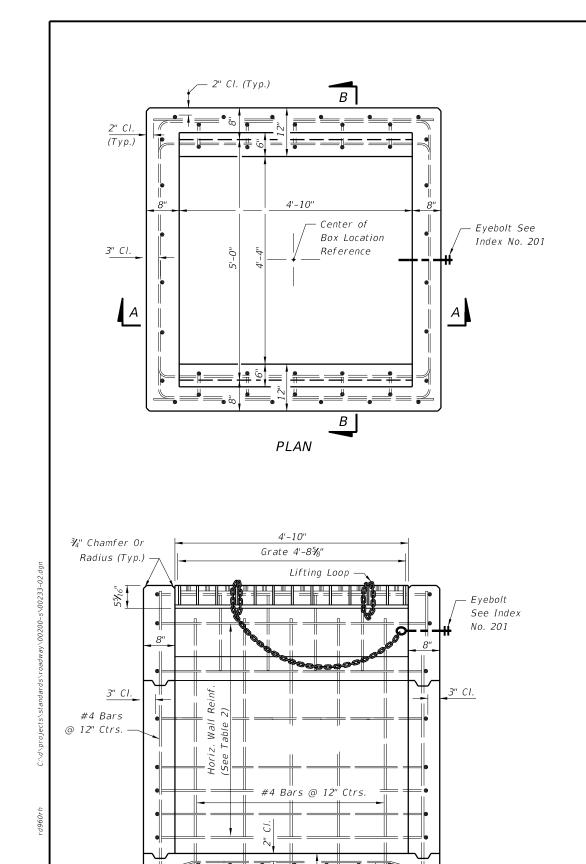
2. Cost of paving to be included in cost of inlet.



FDOT DESIGN STANDARDS 2013

DITCH BOTTOM INLET TYPES F AND G

INDEX SHEET NO.



4'-8½"

(22 Spaces, 23 Bars)

L 5x3½x¾6

SECTION CC

SECTION DD

STEEL GRATE

5" Steel Decking, Weight 630 Lbs. Main Bars 5" x ¼"

Intermediate Bars 1½" x¾", Reticuline Bars 1½" x ¾6"

Grate 5'-3" Grate 5'-3" #4 Bars @ 12" Ctrs. #4 Bars @ 12" Ctrs.

TYPE G INLET (TABLE 2)

WALL	SCHEDULE	AREA	MAX. SPACING								
DEPTH	SCHEDULE	(in²/ft)	BARS	WWF							
0' - 3'	A12	0.20	12"	8"							
3' - 7'	A6	0.20	6"	5"							
7' - 10'	B5.5	0.24	51⁄2"	5"							
10' - 15'	C6.5	0.37	6 ½ "	6"							

TYPE G

└─ #4 Bars @ 8" Ctrs.

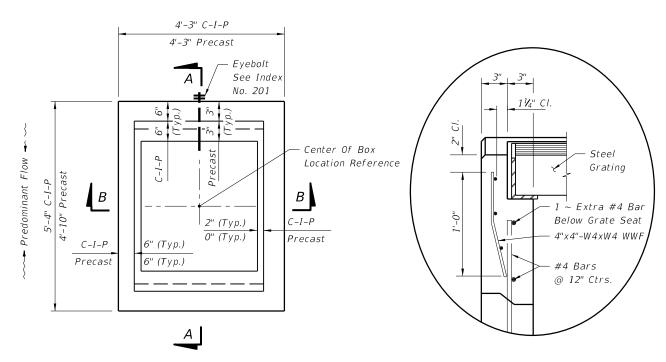
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SECTION AA

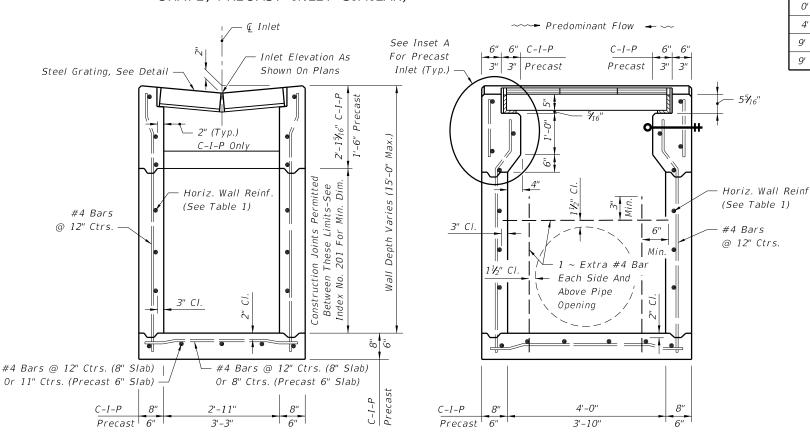


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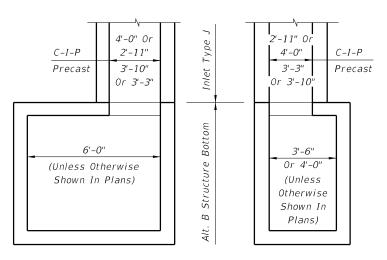
PLAN(CAST-IN-PLACE INLET SHOWN, WITHOUT GRATE, PRECAST INLET SIMILAR)

INSET A (PRECAST OPTION)



(Pipe Opening Not Shown) SECTION BB

(Pipe Opening Shown) SECTION AA



NOTE: Alt. B Structure Bottom Only. See Index No. 200 for structure bottom details and hole reinforcement.

INLET WITH STRUCTURE BOTTOM

RECOMMENDED MAXIMU	JM PIPE SIZES
INLET INSIDE WIDTH	PIPE SIZE
2'-11" or 3'-3"	24"
3'-10" or 4'-0"	30"

Note: Recommended sizes are for concrete pipe. Sizes for other types of pipe must be verified for fit in accordance with Index No. 201. For larger pipe, see Structure Bottom detail above and Index No. 200.

GENERAL NOTES

- 1. This inlet is designed for use in ditches, medians, pavement areas or other areas subject to heavy wheel loads with minimal debris. This inlet is not for use in areas subject to bicycle traffic. This inlet may be placed in areas subject to occasional pedestrian traffic such as landscaped areas and pavement areas where pedestrians can walk around the inlet.
- 2. All reinforcing Grade 60 bars with 2" min. cover unless otherwise noted. See Index No. 201 for equivalent area of welded wire fabric. Cut or bend bars out of way of pipe when necessary; bars to clear pipe by 1½".
- 3. All exposed edges and corners shall be $\frac{3}{4}$ " chamfer or tooled to $\frac{1}{4}$ " radius.
- 4. When alternate G grate is specified in plans the grate is to be hot dip galvanized after fabrication.
- 5. For supplemental details, see Index No. 201.

HORIZONTAL WALL REINFORCING SCHEDULE (TABLE 1)

SCHEDULE

A12

Α6

B5.5

AREA

(In²/ft)

0.20

0.20

0.24

WALL

DEPTH

4' - 9'

9' - 15'

MAX. SPACING

BARS WWF

5½"

5"

3"

5"

- 6. All dimensions are for both precast and cast-in-place inlets unless otherwise noted.
- 7. Cost of ditch paving to be included in cost of inlet. Sodding to be paid for under contract unit price for Performance Turf, SY.

REVISION 07/01/09

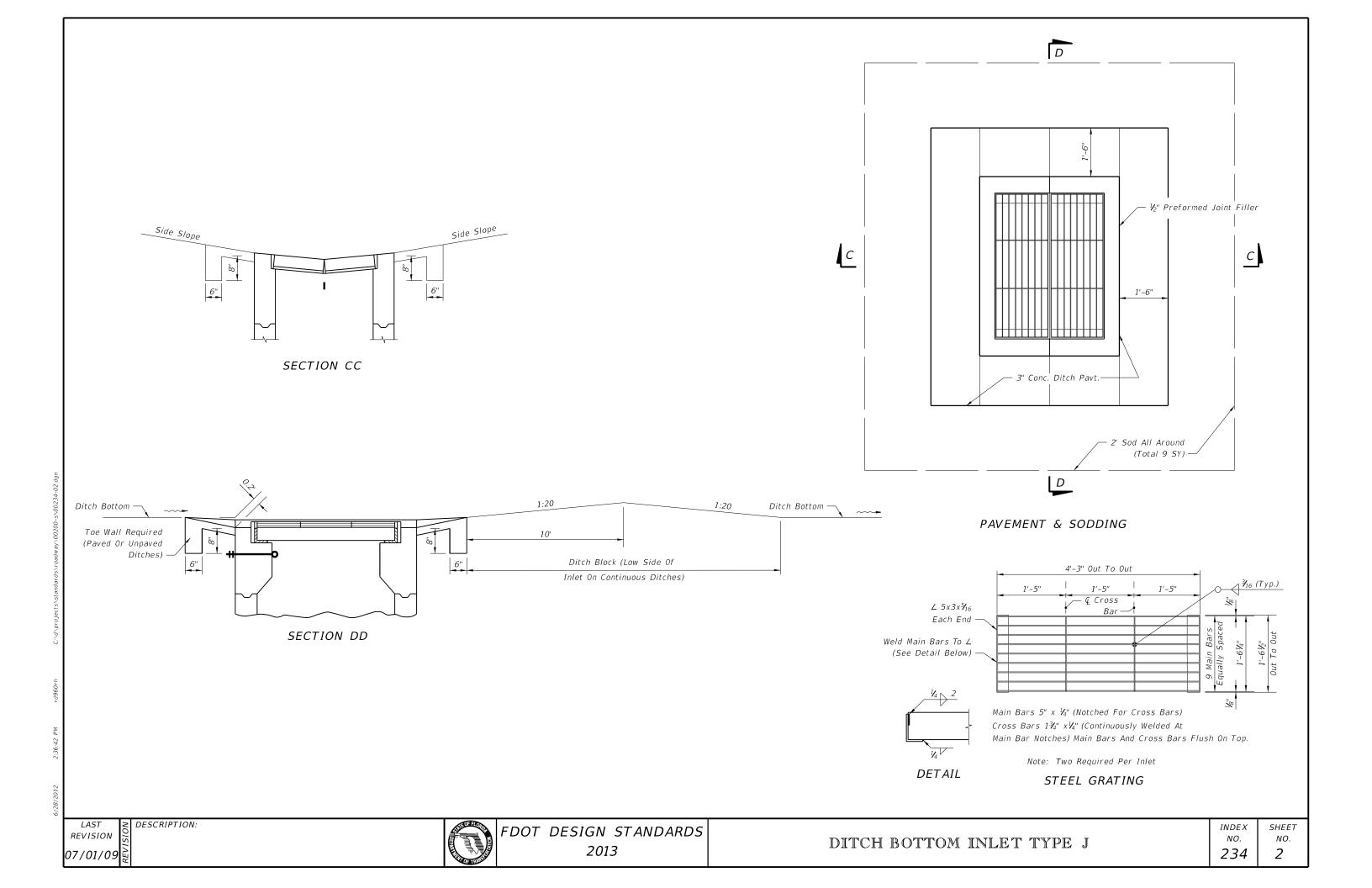
DESCRIPTION:

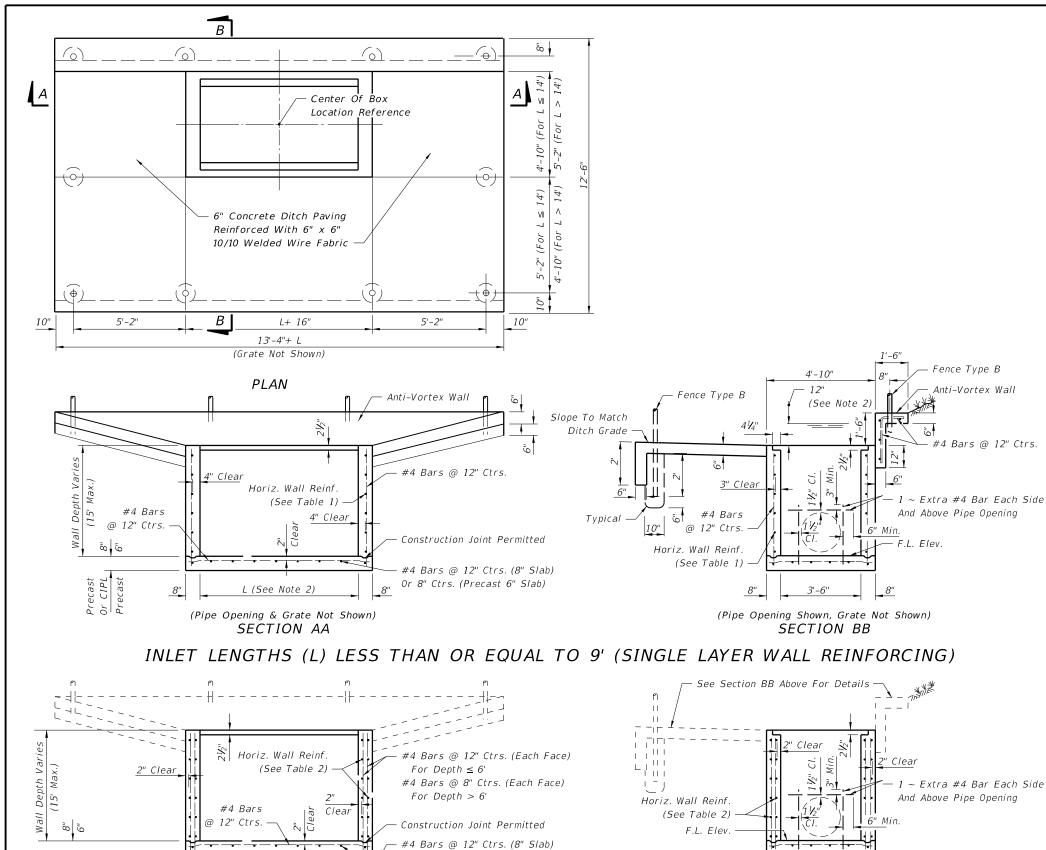


FDOT DESIGN STANDARDS 2013

DITCH BOTTOM INLET TYPE J

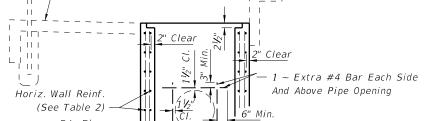
INDEXSHEET NO. NO. 234





GENERAL NOTES

- 1. This inlet is to be used at locations having high flow rates, usually where an endwall could not be utilized without hazardous intake.
- 2. Inlet length (L) shall be set by the designer for the greater of either culvert requirement or inlet pool not to exceed 12" depth. Structures over 6 feet in depth are to be checked for flotation by the designer of project drainage.
- 3. This inlet is not intended for use with Index 200 structure bottoms.
- 4. All exposed edges and corners shall be $rac{3}{4}$ " chamfer or tooled to $rac{1}{4}$ "
- 5. Inlet and anti-vortex wall to be Class II Concrete.
- 6. All reinforcing is Grade 60 with 2" min. cover unless otherwise noted. See Index No. 201 for equivalent area of welded wire fabric (WWF). Bars to be cut or bent for $1\frac{1}{2}$ " clearance around pipe opening. Bend top and corner bars to clear anchor holes.
- 7. Channel section C 3x6 at 14" max. bar spacing may be used as an alternate for the C 4x5.4 channel at 15" bar spacing.
- 8. Channels and bars for grate shall be ASTM A242/A242M, A572/A572M or A588/A588M, Grade 50 steel, and galvanized in accordance with Specification Section 975.
- Fence enclosure shall be Fence Type B (Index No. 802). All posts to be set in concrete. A minimum of 10 posts required. Corner and approach side posts to be 3" nominal diameter.
- 10. Cost of ditch paving, anti-vortex wall, grate, concrete, reinforcing steel and fence enclosure to be included in the cost of inlet. Inlet to be paid for under the contract unit price for Inlets (DT Bot) (Type K), Each.
- 11. Anchor Bolts shall be ASTM F1554 Grade 36 fully threaded headless bolts, installed in accordance with Specification Sections 416 and 937. Nuts shall be ASTM A563 or A194 and washers shall be ASTM F436 or Type A plain washers. All nuts, bolts and washers shall be galvanized.



 $8'' \; (For \; L \leq 14')$

10" (For L > 14')

(Pipe Opening & Grate Not Shown) SECTION AA

L (See Note 2)

(Pipe Opening Shown, Grate Not Shown) SECTION BB

INLET LENGTHS (L) GREATER THAN OR EQUAL TO 9' (DOUBLE LAYER WALL REINFORCING)

DESCRIPTION: LAST REVISION 07/01/09

 $8'' \; (For \; L \leq 14')$

 $10'' \; (For \; L > 14')$



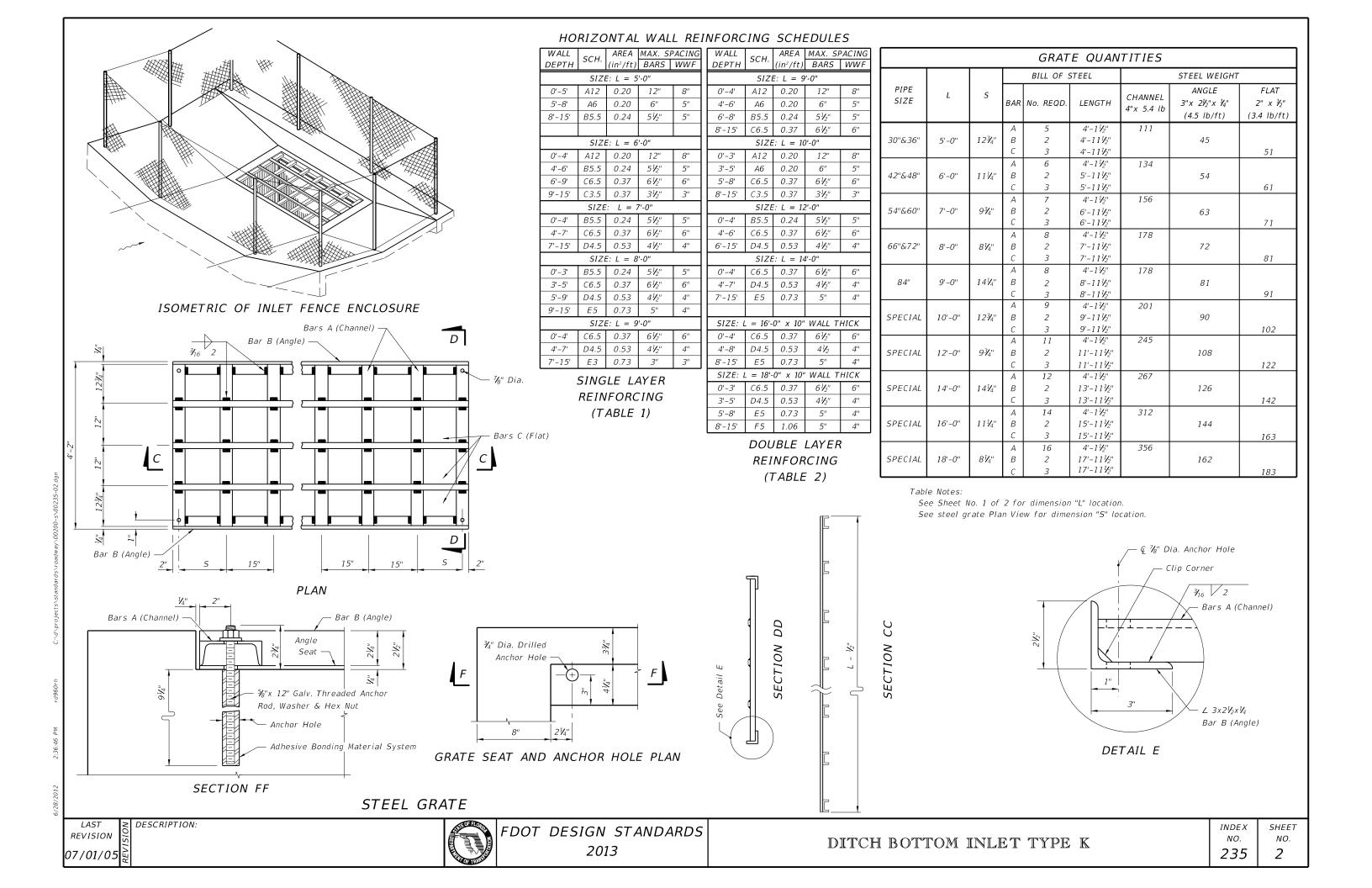
Or 8" Ctrs. (Precast 6" Slab)

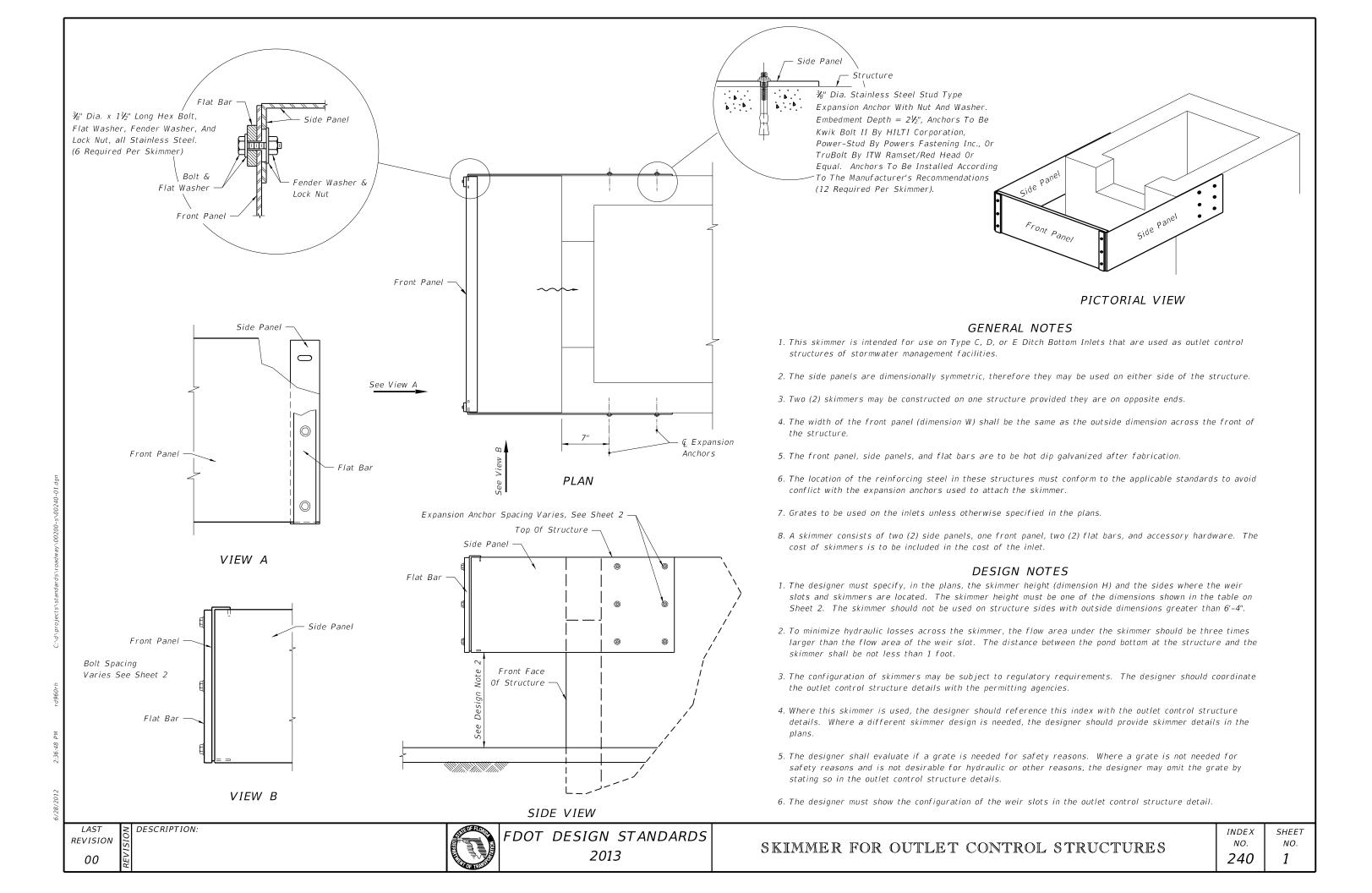
 $8'' \; (For \; L \leq 14')$

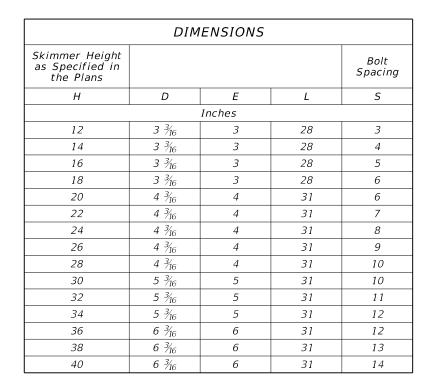
10'' (For L > 14')

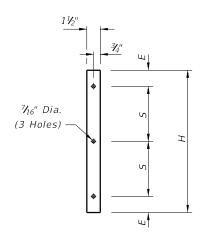
 $8'' \; (For \; L \leq 14')$

10'' (For L > 14')



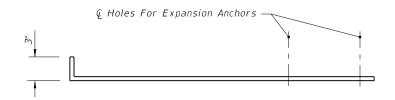




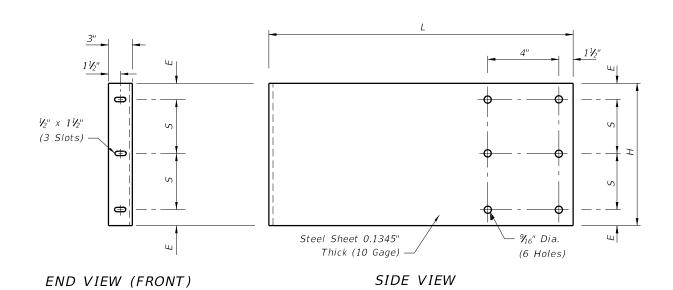


¼" Thick x 1½" Wide

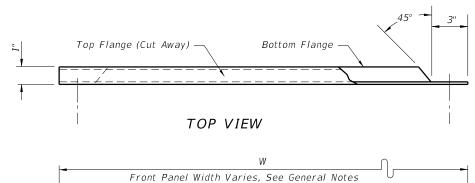
FLAT BAR

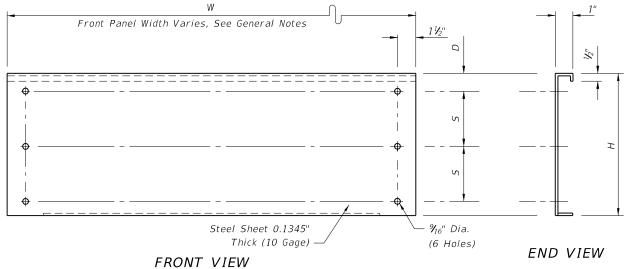


TOP VIEW



SIDE PANEL

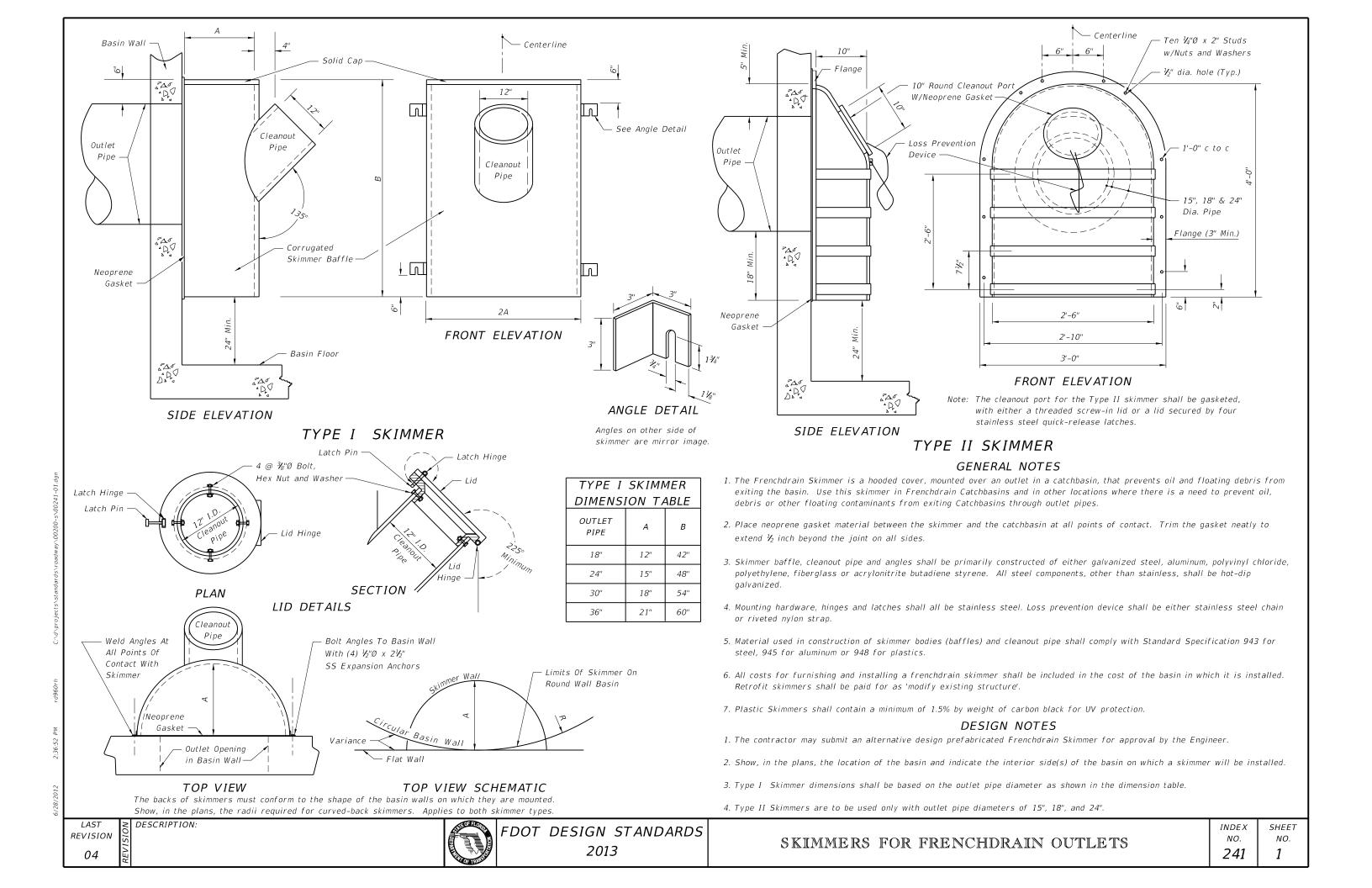


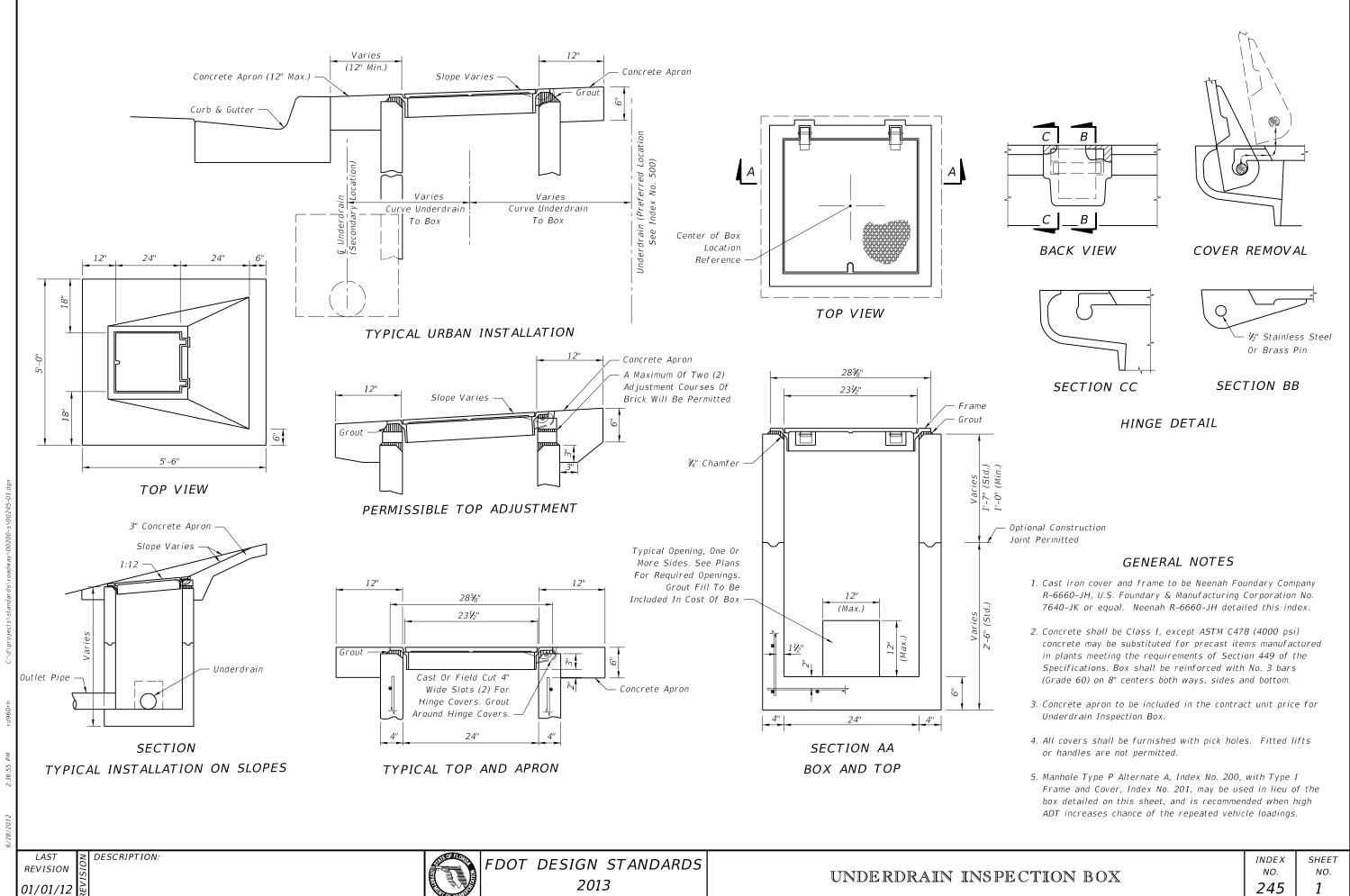


FRONT PANEL

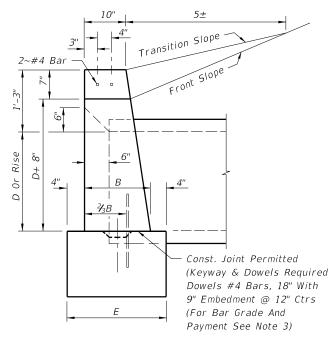
LAST REVISION 01/01/12

DESCRIPTION:



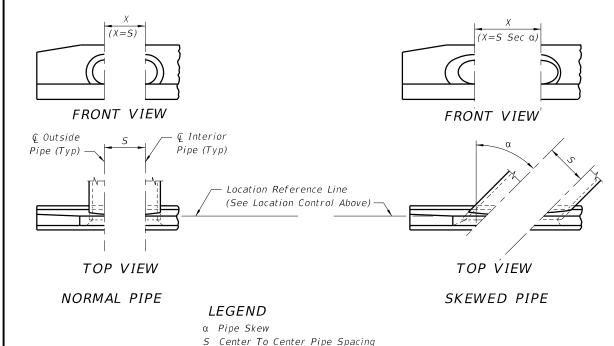


TOP VIEW



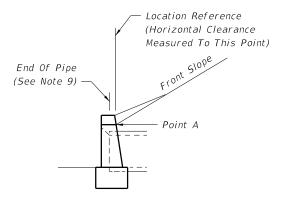
END VIEW (ENLARGED)

ENDWALL DIMENSIONS (EXCLUSIVE OF MULTIPLE PIPE SPACING)



PIPE AND SPACING FOR MULTIPLE PIPE ENDWALL POSITIONS FOR SINGLE AND MULTIPLE

X Centerline To Centerline Dimension At Face Of Headwall



END VIEW

- 1. Position is set by the intersection of the front slope and Point A where this intersection falls outside the clear zone.
- 2. Where the front slope and Point A intersects inside the clear zone, the endwall is positioned so the location reference point is at the clear zone limit. The front slope is transitioned to the endwall as shown in Index No. 280.

STANDARD LOCATION CONTROL

GENERAL NOTES

- 1. Endwall dimensions, locations and positions are for round and elliptical concrete pipe and for round and pipe-arch corrugated metal pipe. Round concrete pipe shown.
- 2. Front slope and ditch transitions shall be in accordance with Index No. 280.
- 3. Endwalls may be cast in place or precast concrete. Reinforcing steel shall be Grades 40 or 60. Additional reinforcement necessary for handling precast units shall be determined by the Contractor or the supplier. Cost of reinforcement shall be included in the contract unit price for Concrete, (Endwalls).
- 4. All exposed corners and edges of concrete are to be chamfered ¾".
- 5. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 6. On outfall ditches with side slopes flatter than $1:1\frac{1}{2}$ provide 20' transitions from the endwall to the flatter side slopes, right of way permitting.
- 7. For sodding around endwalls see Index No. 281.
- 8. Payment for concrete quantities for endwalls skewed to the pipe shall be made on the following basis:

Endwall Skew to Pipe Use Tabulated Value 0° to 5° nº 15° 6° 16° 30° 45°

31° or over

- 9. Pipe length plan quantities shall be based on the pipe end locations shown in the standard location control end view, or lengths based on special endwall locations called for in the
- 10. Payment for pipe in pipe culverts shall be based on plan quantities, adjusted for endwall locations subsequently established by the Engineer.
- 11. Endwalls to be paid for under the contract unit price for Class I Concrete (Endwalls), CY.

LAST REVISION 07/01/09

DESCRIPTION:



ROUND CONCRETE AND CORRUGATED METAL PIPE

		0	penino	~ 4.00																								Class	I Co	ncrete	(CY)												
		U	pening (SF	•							Dimens	sions														Numbe	er And	Туре	Of Pipe	And	Skew	Angle	Of Pi	pe]
D			(3.	,							Difficing	,,,,,,					Si	ngle				Do	uble							Trip	le							Quar	druple				D
		Nun	nber C	Of Pip	es	Λ	R		F	F	G	5			X		Conc	Metal		Con	crete			М	etal			Conci	rete			Ме	tal			Cor	ncrete			M	etal		1
	1		2	3	4	1 ^		C		,		,	0°	15°	30°	45°	0°	0°	0°	15°	30°	45°	0°	15°	30°	45°	<i>0</i> °	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°	0°	15°	30°	45°	1 /
1.5	" 1.2	23 .	2.46	3.69	4.92	1'-11"	1'-2"	4'-0"	1'-10"	1'-2"	0'-6"	2'-7"	2'-7"	2'-8"	3'-0"	3'-8"	1.23	1.24	1.59	1.60	1.65	1.74	1.62	1.63	1.68	1.78	1.94	1.96	2.05 .	2.23	1.99	2.02	2.11	2.30	2.30	2.34	2.47	2.74	2.37	2.41	2.75	2.84	15"
18	" 1.7	77 .	3.54	5.31	7.08	2'-2"	1'-3"	4'-6"	1'-11"	1'-3"	1'-0"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.56	1.59	1.99	2.01	2.06	2.17	2.04	2.06	2.11	2.23	2.43	2.46	2.56 .	2.79	2.51	2.54	2.65	2.89	2.86	2.91	3.06	3.40	2.96	3.01	3.17	3.53	18"
21	" 2.4	41 .	4.82	7.23	9.64	2'-5"	1'-4"	5'-0"	2'-0"	1'-4"	1'-6"	3'-2"	3'-2"	3'-3"	3'-8"	4'-6"	1.97																										21"
24	" 3.:	14	6.28	9.42	12.56	2'-8"	1'-4"	5'-6"	2'-0"	1'-4"	2'-0"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	2.24	2.29	2.82	2.84	2.91	3.06	2.91	2.93	3.01	3.17	3.39	3.43	<i>3.57</i> .	3.87	3.52	3.56	3.71	4.03	3.97	4.03	4.24	4.69	4.14	4.20	4.43	4.91	24"
27	" 3.9	98	7.96	11.94	15.92	2'-11"	1'-5"	6'-0"	2'-1"	1'-5"	2'-6"	3'-10"	3'-10"	4'-0"	4'-5"	5'-5"	2.73																										27"
30	" 4.9	91 :	9.82	14.73	19.64	3'-2"	1'-6"	6'-6"	2'-2"	1'-6"	3'-0"	4'-3"	4'-3"	4'-5"	4'-11"	6'-0"	3.26	3.34	4.13	4.16	4.26	4.49	4.28	4.31	4.43	4.67	4.98	5.04	5.25 .	5.69	5.20	5.27	5.49	5.97	5.84	5.93	6.24	6.91	6.13	6.23	6.56	7.29	30"
36	" 7.0	07 1	4.14	21.21	28.28	3'-8"	1'-8"	7'-6"	2'-4"	1'-8"	4'-0"	5'-1"	5'-1"	5'-3"	5'-10"	7'-2"	4.53	4.64	5.73	5.77	5.92	6.23	5.95	6.00	6.15	6.49	6.92	7.00	7.29	7.91	7.25	7.34	7.65	8.33	8.13	8.26	8.69	9.62	8.57	8.71	9.18	10.20	36"
42	" 9.6	52 1	9.24	28.86	38.48	4'-2"	1'-10"	8'-6"	2'-6"	2'-0"	5'-0"	6'-0"	6'-0"	6'-3"	6'-11"	8'-6"	6.33	6.49	8.11	8.17	8.39	8.85	8.43	8.50	8.73	9.23	9.90	10.02	10.45 1	1.38	10.38	10.52	10.98	11.99	11.68	11.87	12.51	13.89	12.32	2 12.52	13.22	14.73	42"
48	" 12.	57 2	25.14	37.71	50.28	4'-8"	2'-1"	9'-6"	2'-9"	2'-0"	6'-0"	6'-9"	6'-9"	7'-0"	7'-10"	9'-7"	8.15	8.38	10.40	10.48	10.75	11.33	10.85	10.94	11.23	11.87	12.64	12.80	13.34 1	4.50	13.34	13.51	14.11	15.39	14.89	15.13	; 15.93	17.68	15.82	2 16.08	16.97	18.90	48"
54	" 15.	90 3	31.80	47.70	63.60	5'-2"	2'-6"	10'-6"	3'-2"	2'-3"	7'-0"	7'-8"	7'-8"	7'-11"	8'-10"	10'-10"	11.71	11.77	15.23	15.35	15.78	16.69	15.35	15.48	15.90	16.83	18.77	19.02	19.86 2	21.69	18.93	19.18	20.04	21.89	22.29	22.66	i 23.93	26.67	22.51	. 22.89	24.17	26.96	54"

													CORF	RUGA	TED I	METAL	PIPE	ARCI	4													,						
	Opening Area Dimensions														Class I Concrete (CY)															Approx.								
Span	Rise		(5	SF)						_	onnension.	5					Number Of Pipe And Skew Angle Of Pipe													Span	Rise	Equiv. Round						
		Nι	Number Of Pipes			umber Of Pipes			Number Of Pipes						_	_	-				X		Single		Dou	ıble			Tri	ple			Quad	ruple			1 '	Pipe
		1	2	3	4	A	В	(E	<i>F</i>	G	5	0°	15°	30°	45°	0°	0°	15°	30°	45°	0°	15°	30°	45°	<i>0°</i>	15°	30°	45°		1 '							
17"	13"	1.1	2.2	3.3	4.4	1'-9"	1'-2"	3'-10"	1'-10"	1'-2"	0'-4"	2'-6"	2'-6"	2'-7"	2'-11"	3'-6"	1.16	1.47	1.48	1.52	1.60	1.78	1.80	1.88	2.04	2.09	2.12	2.23	2.48	17"	13"	15"						
21"	15"	1.6	3.2	4.8	6.4	1'-11"	1'-2"	4'-3"	1'-10"	1'-2"	0'-9"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.33	1.69	1.70	1.75	1.84	2.04	2.06	2.15	2.33	2.40	2.44	2.57	2.84	21"	15"	18"						
28"	20"	2.8	5.6	8.4	11.2	2'-4"	1'-3"	5'-2"	1'-11"	1'-3"	1'-8"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	1.78	2.31	2.33	2.39	2.53	2.83	2.87	2.99	3.26	3.36	3.42	3.60	4.01	28"	20"	24"						
35"	24"	4.3	8.6	12.9	17.2	2'-8"	1'-4"	5'-11½"	2'-0"	1'-4"	2'-51/2"	4'-0"	4'-0"	4'-2"	4'-7"	5'-8"	2.34	3.03	3.05	3.14	3.32	3.72	3.77	3.93	4.29	4.40	4.47	4.72	5.25	35"	24"	30"						
42"	29"	5.9	11.8	17.7	23.6	3'-1"	1'-5"	6'-10½"	2'-1"	1'-5"	3'-41/2"	4'-9"	4'-9"	4'-11"	5'-6"	6'-9"	3.13	4.06	4.09	4.20	4.45	4.99	5.06	5.28	5.76	5.93	6.03	6.36	7.09	42"	29"	36"						
49"	33"	8.4	16.8	25.2	33.6	3'-5"	1'-6"	7'-8"	2'-2"	1'-6"	4'-2"	5'-6"	5'-6"	5'-8"	6'-4"	7'-9"	3.83	5.00	5.04	5.18	5.48	6.16	6.24	6.52	7.12	7.32	7.44	7.86	8.76	49"	33"	42"						
57"	38"	10.6	21.2	31.8	42.4	3'-10"	1'-7"	8'-7 ¹ / ₂ "	2'-3"	1'-7"	5'-1 ¹ / ₂ "	6'-4"	6'-4"	6'-7"	7'-4"	8'-11"	4.87	6.31	6.36	6.53	6.91	7.74	7.84	8.18	8.93	9.18	9.33	9.85	10.96	57"	38"	48"						
64"	43"	13.2	26.4	39.6	52.8	4'-3"	1'-8"	9'-6½"	2'-4"	1'-8"	6'-0 ¹ / ₂ "	7'-1"	7'-1"	7'-4"	8'-2"	10'-0"	5.88	7.64	7.70	7.91	8.37	9.40	9.52	9.94	10.86	11.15	11.33	11.97	13.33	64"	43"	54"						
71"	47"	16.9	33.8	50.7	67.6	4'-7"	1'-10"	10'-4"	2'-6"	2'-0"	6'-10"	7'-10"	7'-10"	8'-1"	9'-1"	11'-1"	7.80	10.15	10.23	10.51	11.12	12.49	12.65	13.22	14.43	14.85	15.10	15.94	17.77	71"	47"	60"						

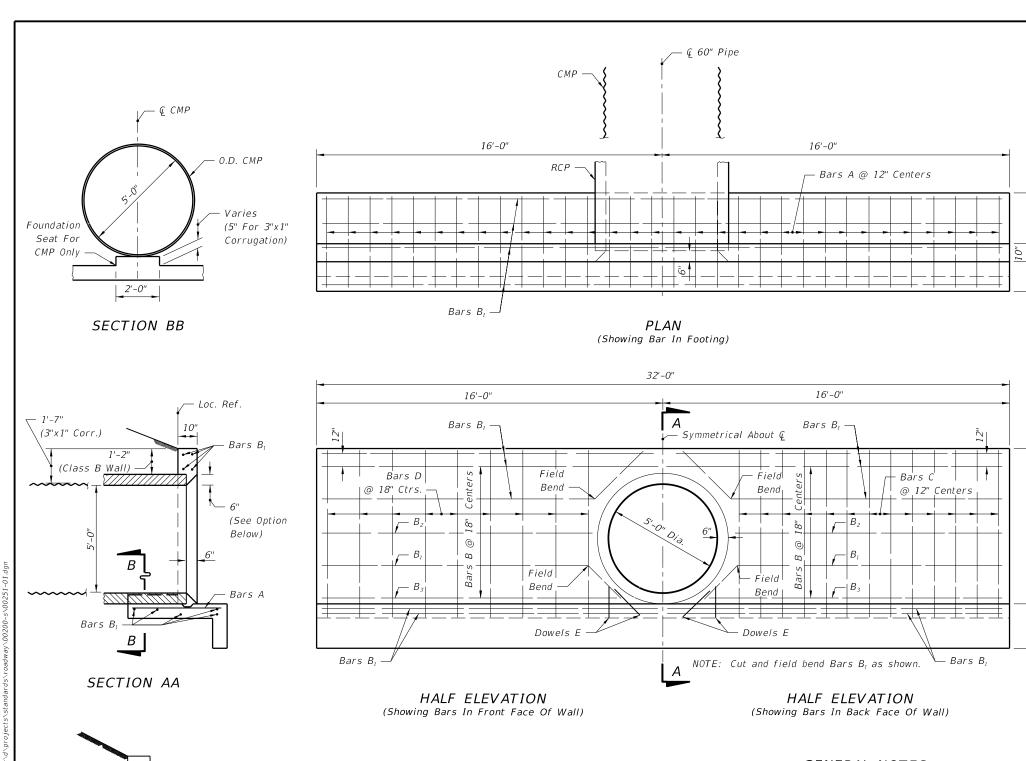
Note: Use the guidelines of General Note No. 8 for selecting tabular quantities.

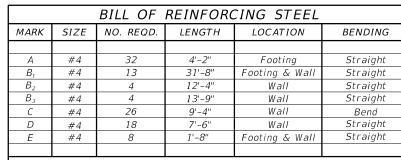
													CC	NCRE	TE EL	LLIPT	ICAL P	PIPE														
Rise	Span	Opening Area (SF)				Dimensions											Class I Concrete (CY) Number Of Pipe And Skew Angle Of Pipe										Rise	Span	Approx. Equiv.			
71756		Number Of Pipes								G		Х		Single	Double			Triple				Quadruple				156	''	Round Pipe				
		1	2	3	4	A	В		E	<i>F</i>	G	5	0°	15°	30°	45°	0°	0°	15°	30°	45°	0°	15°	30°	45°	<i>0°</i>	15°	30°	45°			
12"	18"	1.3	2.6	3.9	5.2	1'-8"	1'-2"	3'-9"	1'-10"	1'-2"	0'-3"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.09	1.45	1.46	1.51	1.60	1.80	1.82	1.91	2.09	2.16	2.20	2.33	2.60	12"	18"	15"
14"	23"	1.8	3.6	5.4	7.2	1'-10"	1'-3"	4'-21/2"	1'-11"	1'-3"	81/2"	3'-5"	3'-5"	3'-6"	3'-11"	4'-10"	1.36	1.82	1.84	1.89	2.01	2.29	2.32	2.43	2.68	2.75	2.80	2.97	3.33	14"	23"	18"
19"	30"	3.3	6.6	9.9	13.2	2'-3"	1'-4"	5'-1½"	2'-0"	1'-4"	1'-7½"	4'-2"	4'-2"	4'-4"	4'-10"	5'-11"	1.89	2.55	2.57	2.65	2.82	3.22	3.27	3.43	3.77	3.88	3.95	4.19	4.70	19"	30"	24"
24"	38"	5.1	10.2	15.3	20.4	2'-8"	1'-5"	6'-3"	2'-1"	1'-5"	2'-9"	5'-2"	5'-2"	5'-4"	6'-0"	7'-4"	2.64	3.55	3.58	3.69	3.93	4.48	4.54	4.77	5.24	5.39	5.49	5.82	6.53	24"	38"	30"
29"	45"	7.4	14.8	22.2	29.6	3'-1"	1'-6"	7'-0"	2'-2"	1'-6"	3'-6"	6'-0"	6'-0"	6'-3"	6'-11"	8'-6"	3.32	4.48	4.52	4.66	4.96	5.64	5.72	6.00	6.60	6.80	6.92	7.34	8.24	29"	45"	36"
34"	53"	10.2	20.4	30.6	40.8	3'-6"	1'-7"	7'-11½"	2'-3"	1'-7"	4'-5½"	7'-1"	7'-1"	7'-4"	8'-2"	10'-0"	4.24	5.76	5.81	6.00	6.39	7.29	7.40	7.76	8.55	8.81	8.97	9.52	10.70	34"	53"	42"
38"	60"	12.9	25.8	38.7	51.6	3'-10"	1'-8"	8'-9"	2'-4"	1'-8"	5'-3"	7'-11"	7'-11"	8'-2"	9'-2"	11'-2"	5.22	7.16	7.23	7.46	7.96	9.10	9.24	9.70	10.71	11.05	11.25	11.95	13.46	38"	60"	48"
43"	68"	16.6	33.2	49.8	66.4	4'-3"	1'-10"	9'-81/2"	2'-6"	1'-10"	6'-2 ¹ / ₂ "	8'-10"	8'-10"	9'-2"	10'-2"	12'-6"	6.63	9.01	9.09	9.38	10.00	11.39	11.56	12.13	13.36	13.77	14.02	14.88	16.73	43"	68"	54"
48"	76"	20.5	41.0	61.5	82.0	4'-8"	2'-1"	10'-8"	2'-9"	2'-0"	7'-2"	9'-9"	9'-9"	10'-1"	11'-3"	13'-9"	8.66	11.74	11.85	12.22	13.02	14.82	15.04	15.77	17.37	17.91	18.23	19.34	21.74	48"	76"	60"
53"	83"	24.8	49.6	74.4	99.2	5'-1"	2'-6"	11'-7"	3'-2"	2'-6"	8'-1"	10'-7"	10'-7"	10'-11"	12'-3"	15'-0"	12.50	16.98	16.98	17.67	18.83	21.47	21.78	22.86	25.18	25.97	26.44	28.06	31.55	53"	83"	66"
58"	91"	29.5	59.0	88.5	118.0	5'-6"	2'-10"	12'-6½"	3'-6"	2'-10"	9'-01/2"	11'-4"	11'-4"	11'-9"	13'-1"	16'-0"	16.46	22.26	22.46	23.16	24.66	28.05	28.46	29.85	32.85	33.85	34.46	36.55	41.05	58"	91"	72"
																														1 '	1 1	<i>i</i> '

REVISION 07/01/01

DESCRIPTION:





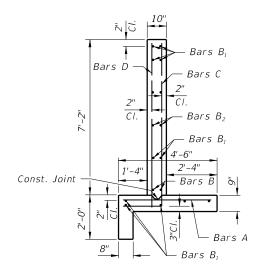


BENDING DIAGRAM



NOTE: All bar dimensions are out to out

ESTIMATED QUANTITIES												
ITEM	UNIT	RCP	CMP									
Class II Concrete	Cu. Yd.	11.3	11.4									
Reinforcing Steel	Lb.	695	695									



TYPICAL SECTION THRU ENDWALL

GENERAL NOTES

- 1. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this index, design specifications AASHTO 1989. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index No. 201 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

- 5. Chamfer: All exposed edges and corners to be chamfered $rac{N}{2}$ unless otherwise shown.
- 6. That portion of corrugated metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of .004" minimum thickness applied prior to placing of the concrete.
- 7. Sodding shall be in accordance with Index No. 281 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

DESCRIPTION: REVISION 07/01/09



FDOT DESIGN STANDARDS 2013

STRAIGHT CONCRETE ENDWALLS

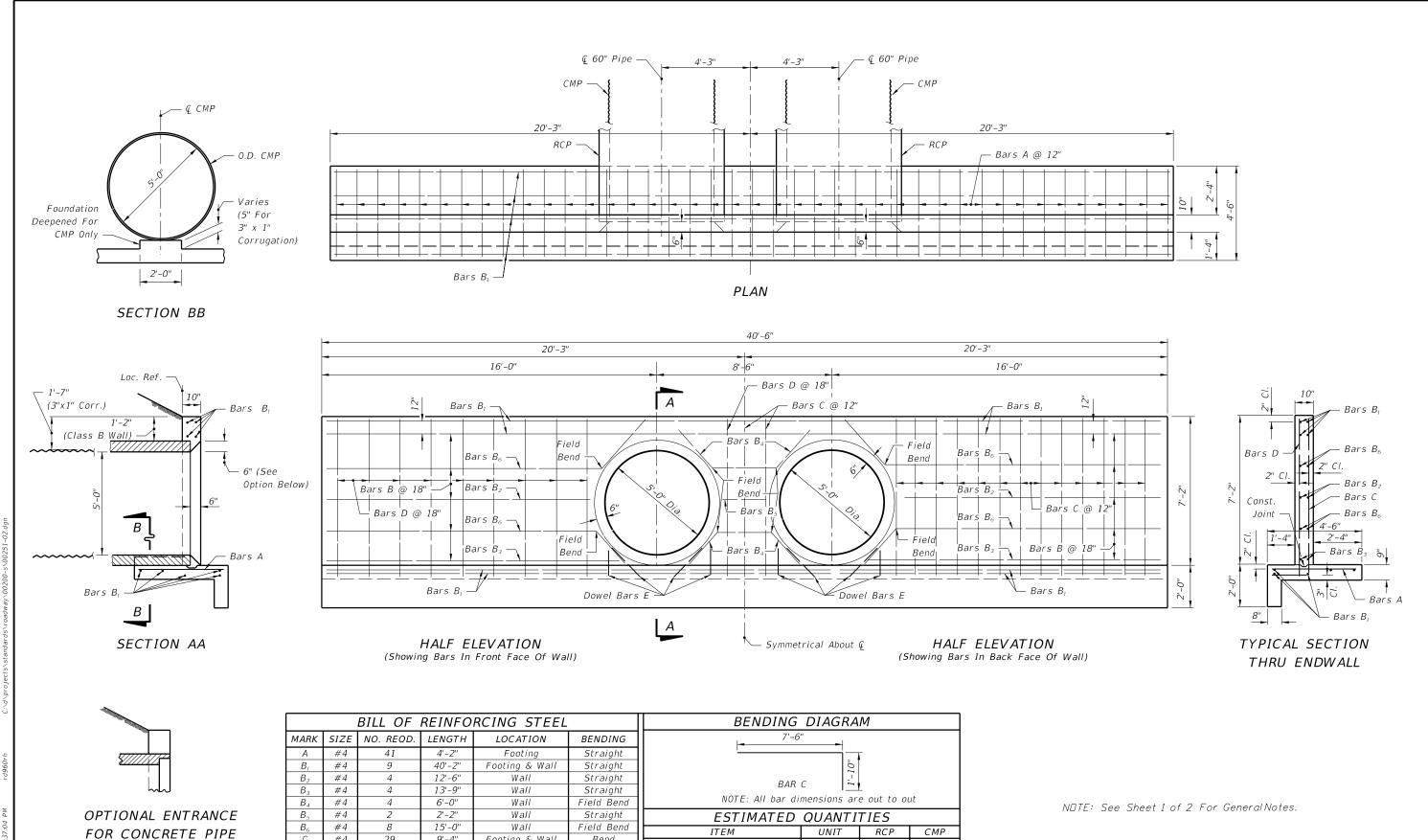
INDEXNO. 251

SHEET NO.

OPTIONAL ENTRANCE

FOR CONCRETE PIPE

SINGLE AND DOUBLE 60" PIPE



LAST REVISION 00

DESCRIPTION:

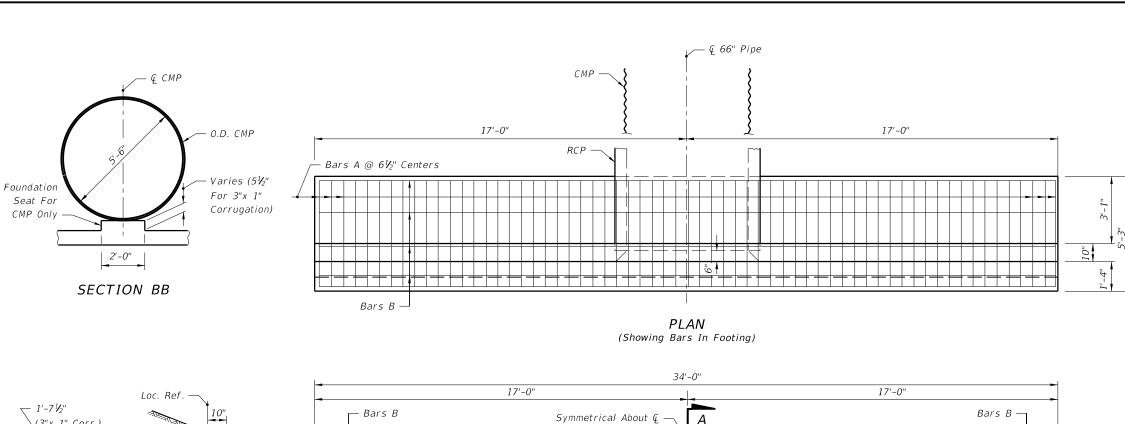


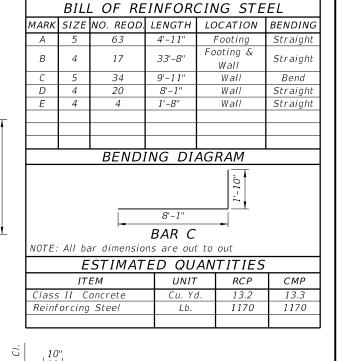
CMP ITEM UNIT RCP 13.7 13.8 Class II Concrete Cu. Yd. 824 824

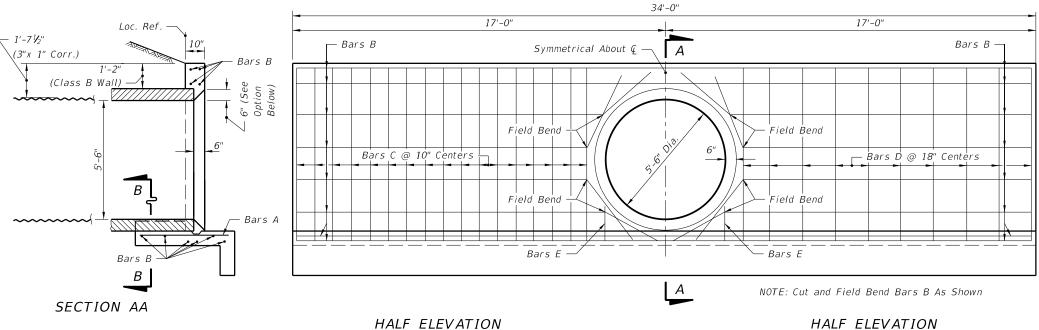
FDOT DESIGN STANDARDS 2013

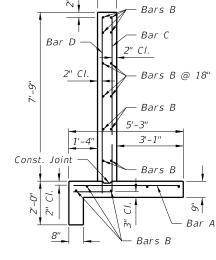
STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 60" PIPE

INDEX NO. 251









HALF ELEVATION (Showing Bars In Front Face Of Wall)

TYPICAL SECTION THRU ENDWALL

GENERAL NOTES

1. Straight concrete endwalls are intended for use outside the clear zone.

(Showing Bars In Back Face Of Wall)

- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this index, design specifications AASHTO 1989. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index No. 201 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

- 5. Chamfer: All exposed edges and corners to be chamfered $rac{N}{2}$ " unless otherwise shown.
- 6. That portion of corrugated Metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of 0.004" minimum thickness applied prior to placing of the concrete.
- 7. Sodding shall be in accordance with Index No. 281 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

LAST REVISION 07/01/09

OPTIONAL ENTRANCE

FOR CONCRETE PIPE

DESCRIPTION:

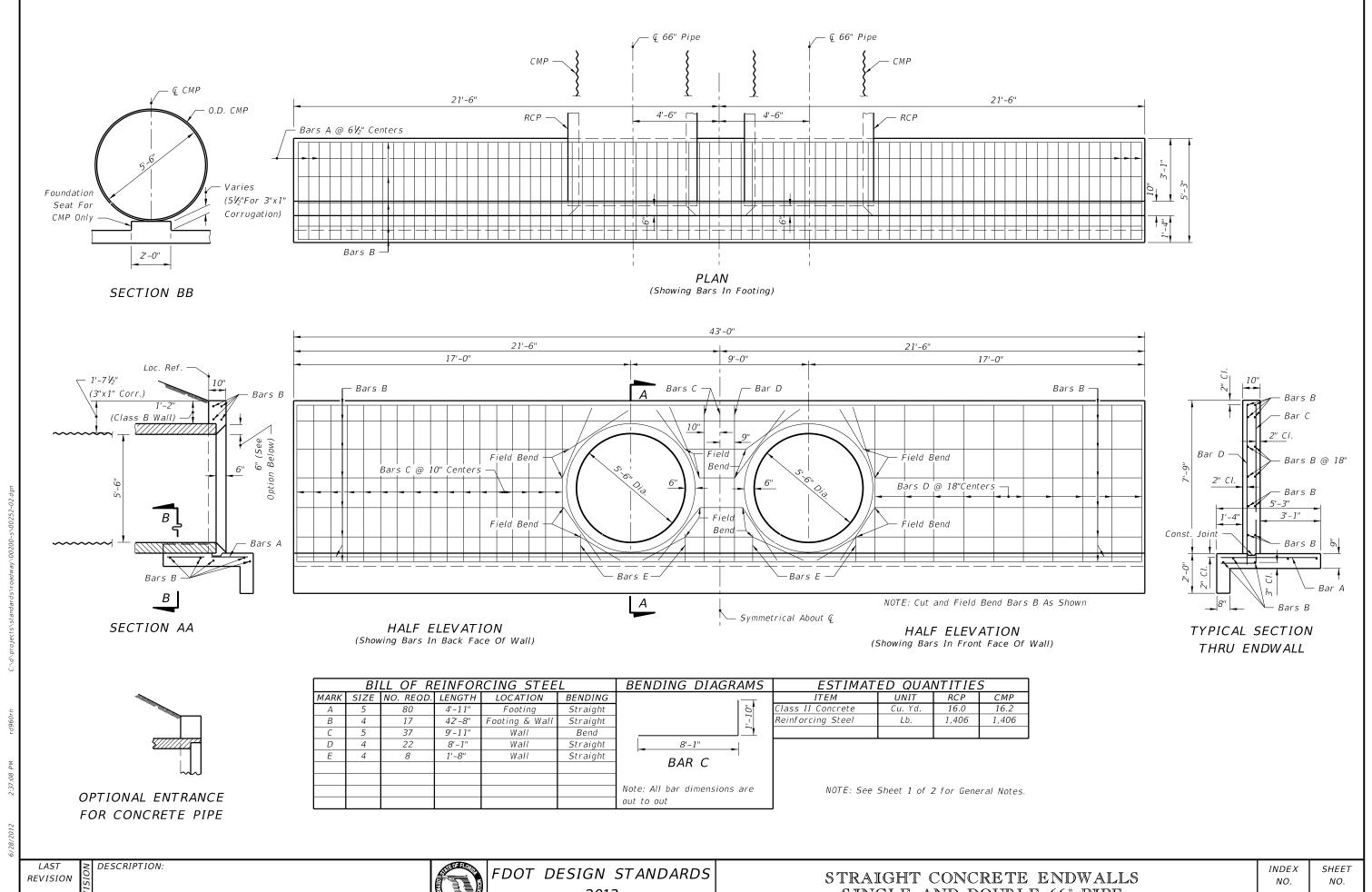


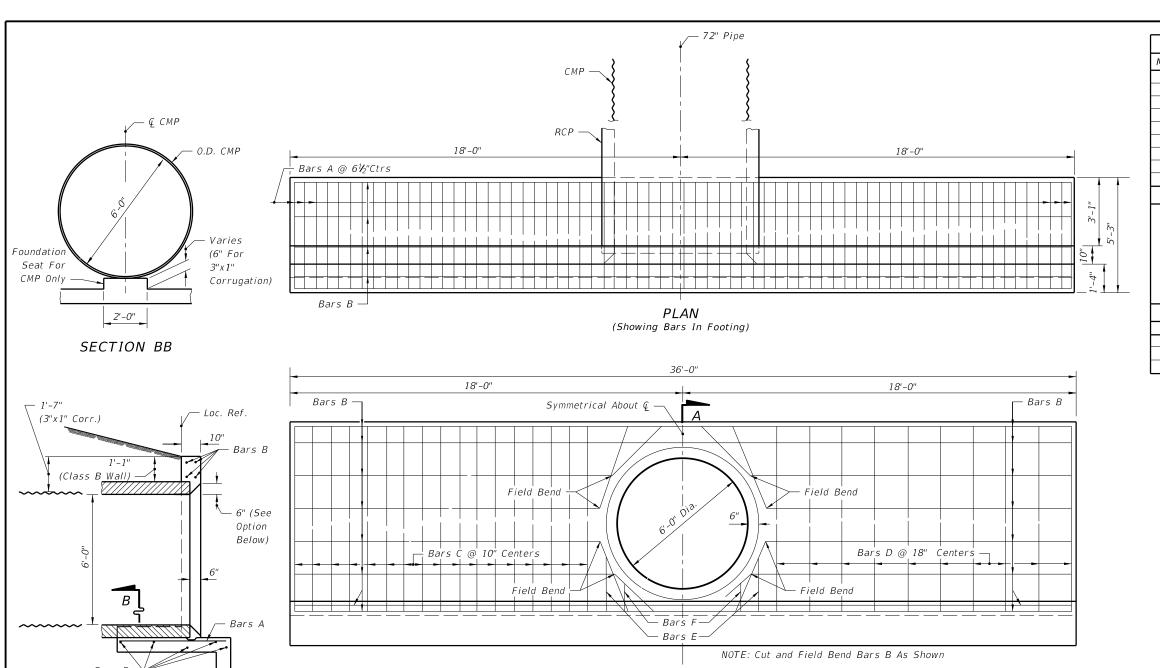
FDOT DESIGN STANDARDS 2013

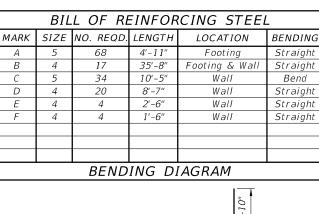
STRAIGHT CONCRETE ENDWALLS

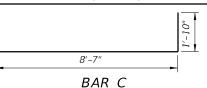
SHEET INDEXNO. NO. 252

SINGLE AND DOUBLE 66" PIPE



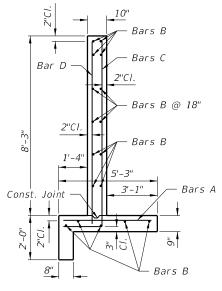






NOTE: All bar dimensions are out to out

ESTIMATE	D QUAN	TITIES	
ITEM	UNIT	RCP	CMP
Class II Concrete	Cu. Yd.	14.4	14.5
Reinforcing Steel	Lb.	1249	1249



TYPICAL SECTION
THRU ENDWALL

GENERAL NOTES

HALF ELEVATION

(Showing Bars In Front Face Of Wall)

- 1. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this index, design specifications AASHTO 1989. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index No. 201 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.

HALF ELEVATION

(Showing Bars In Back Face Of Wall)

4. Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.

- 5. Chamfer: All exposed edges and corners to be chamfered ¾" unless otherwise shown.
- 6. That portion of corrugated Metal pipe in direct contact with the concrete slab and extending 12" beyond shall have a continuous bituminous coating of 0.004" minimum thickness coated applied prior to placing of the concrete.
- 7. Sodding shall be in accordance with Index No. 281 and paid for under the contract unit price for Performance Turf, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Class II Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB.

LAST REVISION



FDOT DESIGN STANDARDS 2013

STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 72" PIPE

INDEX SHEET NO. NO. 253 1

ETE ENDWALLS

OUBLE 72" PIPE

253

C:\d\projects\standar

SECTION AA

OPTIONAL ENTRANCE

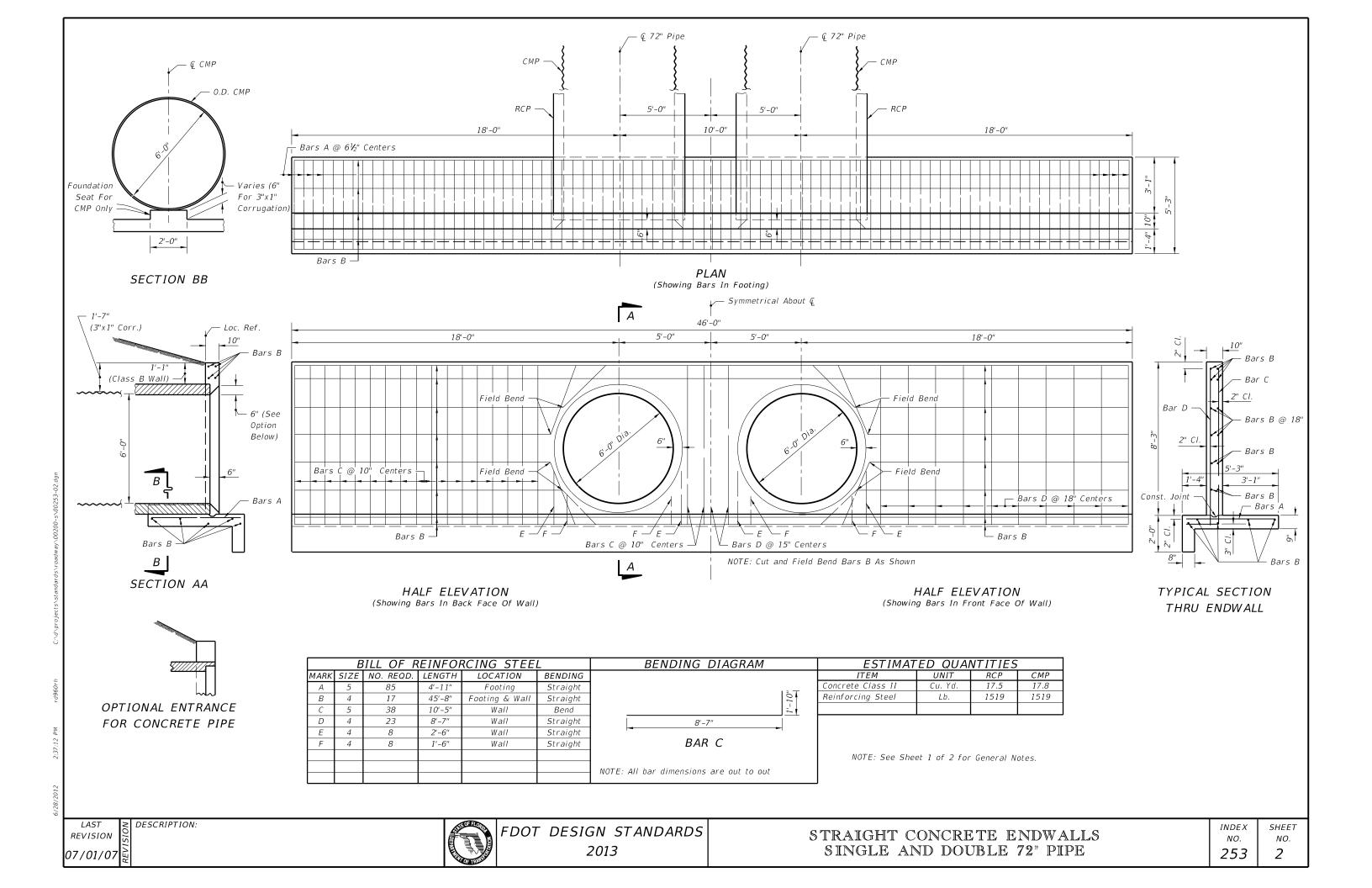
FOR CONCRETE PIPE

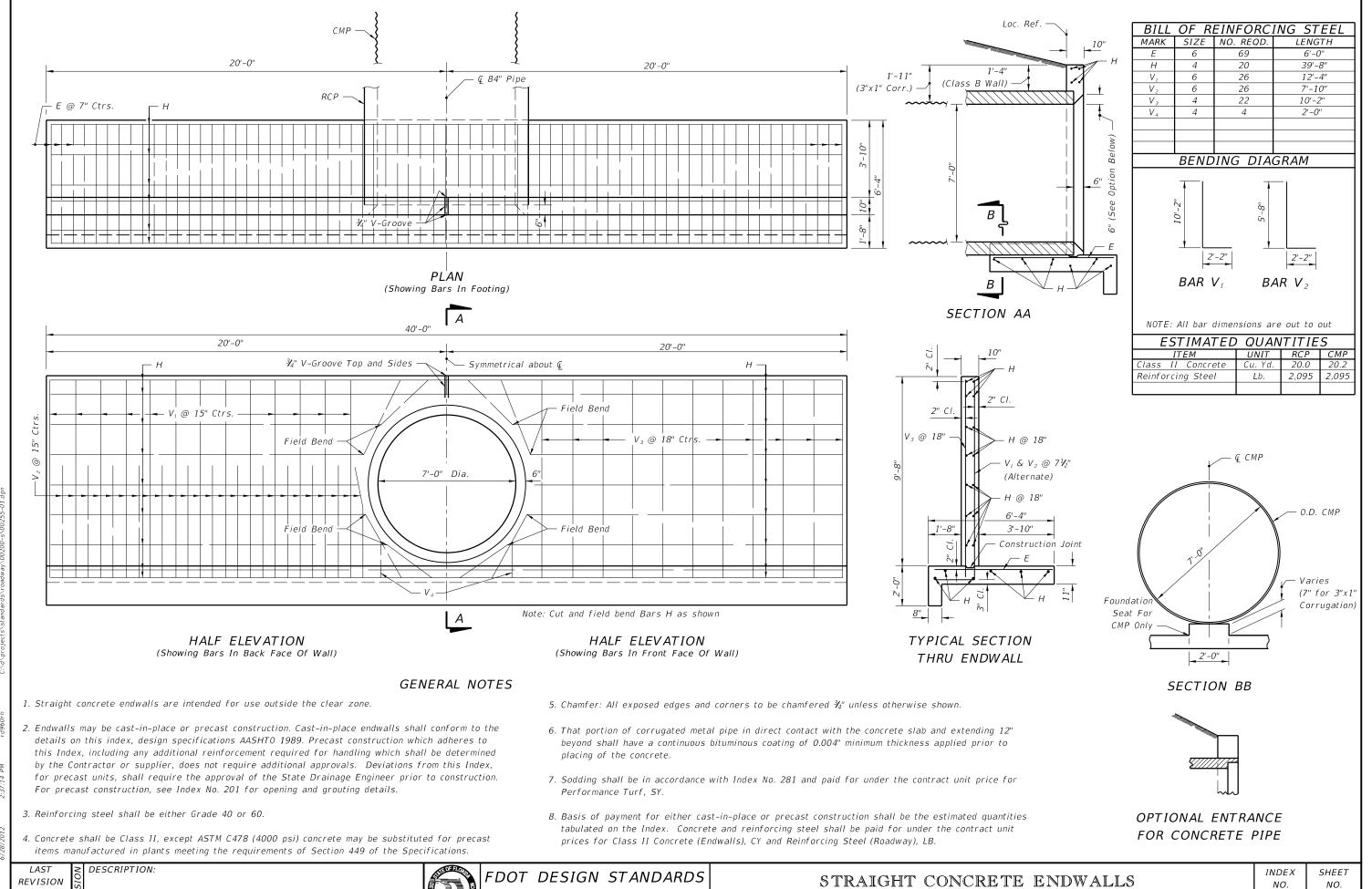
DESCRIPTION:

rd960rh

2:37:10 PM

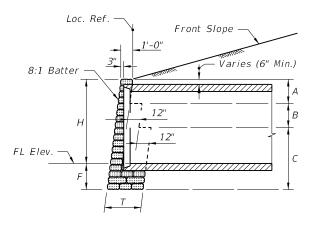
6/28/2012





07/01/09

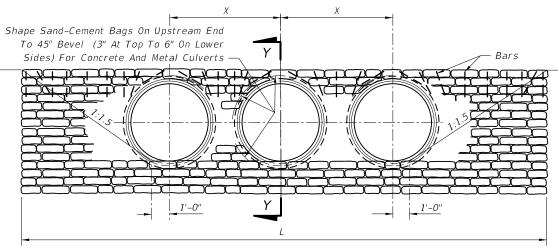




CORRUGATED METAL PIPE

CONCRETE PIPE

SECTION YY



Nata

- 1. For concrete and corrugated metal pipes. Concrete pipe shown.
- 2. The top row of riprap bags shall be secured by pinning, using #4 reinforcing bars 18 inches in length, as follows:
- a. The end bags shall be secured using two bars per bag, one vertical and one diagonal as shown.
- b. The next to last bag on each end shall be secured with two bars vertically.
- c. Bags located over the pipe shall be secured by a bar which is driven diagonally except that for concrete pipe two bars shall be used for single bags above the pipe.
- d. Intermediate bags shall be secured with a single bar.

Bars shall be driven to one inch below the surface of the bag. The cost of furnishing and installing the bars shall be included in the cost of the riprap.

FRONT ELEVATION

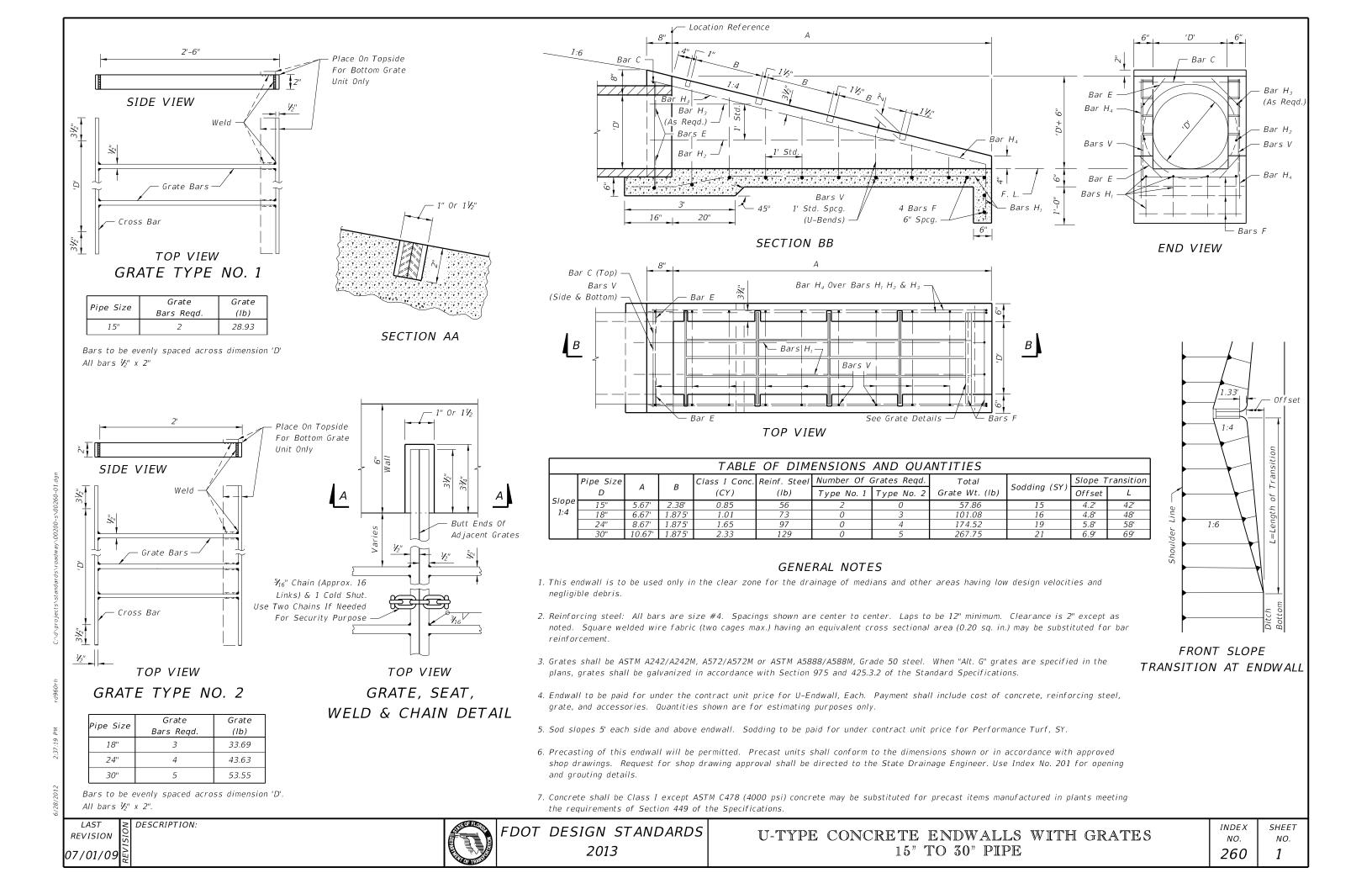
					TABLE	OF	DIMEN	SIONS A	ND	QUAN	TITIES F	OR C	ONE	ENDWALL					
SIZE								ONE PIPE	CULVE	RTS	TWO PIPE	CULVE	RTS	THREE PII	PE CULVE	RTS	FOUR I	PIPE CULV	/ERTS
OF	Н	Τ	Α	В	С	F	X	,	RIPRA	AP CY	,	RIPRA	AP CY	,	RIPRA	AP CY	1	RIPR.	AP CY
PIPE								L	CP	CMP	L	CP	CMP	L	CP	СМР	L	CP	СМР
18"	2'-3"	1'-0"	4'-0"	0'-0"	0'-0"	1'-9"	2'-10"	8'-9"	1.2	1.2	11'-7"	1.5	1.6	14'-5"	1.8	1.9	17'-3"	2.1	2.3
24"	2'-9"	2'-0"	2'-0"	2'-6"	0'-0"	1'-9"	3'-5"	10'-3"	2.4	2.5	13'-8"	3.0	3.2	17'-1"	3.7	4.0	20'-6"	4.3	4.7
30"	3'-4"	2'-0"	2'-0"	3'-2"	0'-0"	1'-10"	4'-3"	12'-0"	3.3	3.4	16'-3"	4.2	4.5	20'-6"	5.1	5.5	24'-9"	6.0	6.5
36"	3'-10"	2'-0"	2'-0"	3'-8"	0'-0"	1'-10"	5'-1"	13'-6"	4.0	4.2	18'-7"	5.2	5.7	23'-8"	6.3	6.9	28'-9"	7.4	8.2
42"	4'-5"	3'-0"	2'-0"	2'-0"	2'-4"	1'-11"	6'-0"	15'-3"	6.4	6.7	21'-3"	8.3	8.9	27'-3"	10.2	11.2	33'-3"	12.3	13.4
48"	4'-11"	3'-0"	2'-0"	2'-0"	2'-10"	1'-11"	6'-9"	16'-9"	7.7	8.1	23'-6"	10.0	10.8	30'-3"	12.3	13.5	37'-0"	14.5	16.2
54"	5'-6"	3'-0"	2'-0"	2'-0"	3'-6"	2'-0"	7'-8"	18'-6"	9.5	10.1	26'-2"	12.4	13.5	33'-10"	15.3	17.0	41'-6"	18.2	20.4
60"	6'-0"	3'-0"	2'-0"	2'-0"	4'-0"	2'-0"	8'-6"	20'-0"	11.0	11.7	28'-6"	14.4	15.8	37'-0"	17.8	19.8	45'-6"	21.1	23.8
66"	6'-7"	3'-0"	2'-0"	2'-0"	4'-8"	2'-1"	9'-3"	21'-9"	13.2	14.1	31'-0"	17.2	18.9	40'-3"	21.2	23.7	49'-6"	25.1	28.5
72"	7'-1"	3'-0"	2'-0"	2'-0"	5'-2"	2'-1"	10'-0"	23'-3"	15.0	16.0	33'-3"	19.4	21.4	43'-3"	23.9	26.8	53'-3"	28.3	32.3
78"	7'-8"	3'-0"	2'-0"	2'-0"	5'-10"	2'-2"	10'-9"	25'-0"	17.5	18.7	35'-9"	22.6	25.0	46'-6"	27.8	31.3	<i>57'-3</i> "	32.9	37.6
84"	8'-2"	3'-0"	2'-0"	2'-0"	6'-4"	2'-2"	11'-8"	26'-6"	19.5	20.9	38'-2"	25.3	28.1	49'-10"	31.1	35.2	61'-6"	36.9	42.4

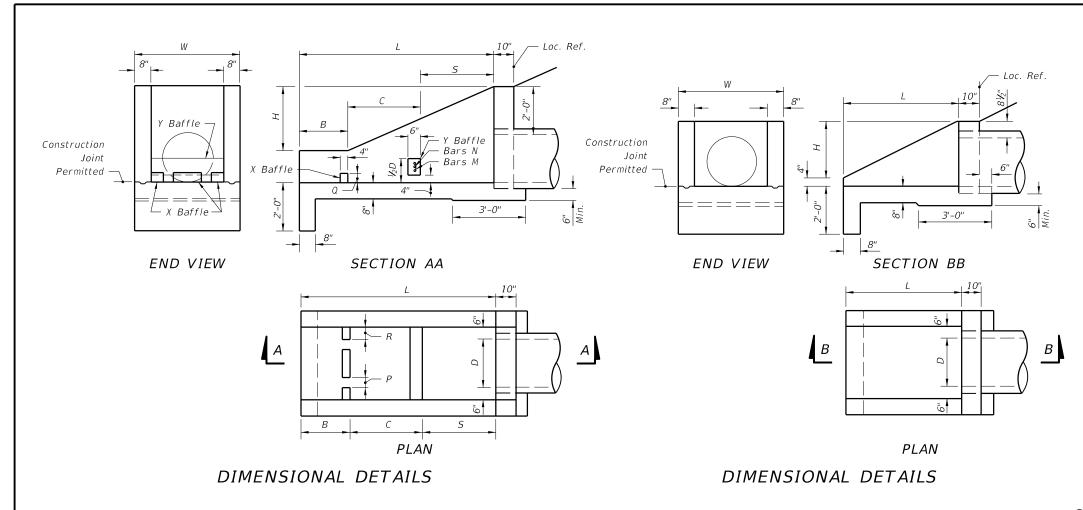
GENERAL NOTES

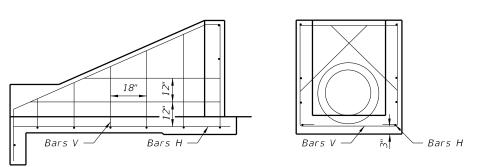
1. Straight sand-cement endwalls are intended for use outside the clear zone.

LAST REVISION 07/01/07









ALL PIPE SIZES SIDE VIEW AND BACKWALL SECTION REINFORCING DETAIL

		D	IMENS.	IONS /	AND (QUAN	TITIES	FC)R	ON	E U-EI	VDW AL	_L	
Pipe	e Size	,	Н	w	S	В		Х	Baft	le	Y Ba Reinf.		Class I Conc.	Reinf. Steel
D	Area Sq. Ft.	L	''	, vv	3	В		P	Q	R	Bar M	Bar N	Cu. Yd.	Lbs.
15"	1.23	5'-9"	2'-31/2"	3'-7"	2'-3"	1'-3"	2'-3"	4"	4"	4"	2 #4	1 #4	1.61	72
18"	1.77	6'-6"	2'-5"	3'-10"	2'-6"	1'-6"	2'-6"	4"	4"	5"	3 #4	2 #4	1.89	86
24"	3.14	8'-0"	2'-8"	4'-4"	3'-0"	2'-0"	3'-0"	5"	5"	6"	4 #4	3 #4	2.52	108
30"	4.91	9'-6"	2'-11"	4'-10"	3'-6"	2'-6"	3'-6"	5"	.5"	7"	4 #4	4 #4	3.34	1.31

ALL PIPE SIZES SIDE VIEW AND BACKWALL SECTION REINFORCING DETAIL

DIMENS	SIONS A	AND QU	ANIIIIE	S FOR C	ONE U-EI	VDW ALL
Pipe	Size				Class I	Reinf.
D	Area Sq. Ft.	L	Н	W	Conc. Cu. Yd.	Steel Lbs.
15"	1.23	3'-3"	1'-7½"	3'-7"	0.89	39
18"	1.77	3'-9"	1'-10½"	3'-10"	1.05	43
24"	3.14	4'-9"	2'-41/2"	4'-4"	1.40	55
30"	4.91	5'-9"	2'-101/2"	4'-10''	1.88	64

WITHOUT BAFFLES WITH BAFFLES

GENERAL NOTES

- 1. Baffles to be constructed only when called for in plans.
- 2. When steel grating is required on endwall see Sheet 3 of 3 for
- 3. All reinforcing No. 4 bars with 2" clearance except as noted.
- 4. All angles, channels and bars shall be ASTM A242/A242M, A572/A572M or A588/A588M Grade 50 steel. When designated Alternate G in the plans galvanize in accordance with Section 975 and 425-3.2 of the Standard Specifications.
- 5. Channel section C 3x6 may be substituted for C 4x5.4 channel.
- 6. Precasting of this endwall will be permitted. Precast units shall conform to the dimensions shown or in accordance with approved shop drawings. Request for shop drawing approval shall be directed to the State Drainage Engineer. Use Index No. 201 for opening and grouting
- 7. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 8. Sodding shall be in accordance with Index No. 281, and paid for under the contract unit price for Performance Turf, SY.
- 9. Endwall to be paid for under the contract unit price for U-Endwall, Each. Payment shall include cost of concrete, reinforcing steel, and when called for in the plans, steel grating, baffles and accessories. Quantities shown are for estimating purposes only.

ENDWALLS FOR 1:2 SLOPES

DESCRIPTION:

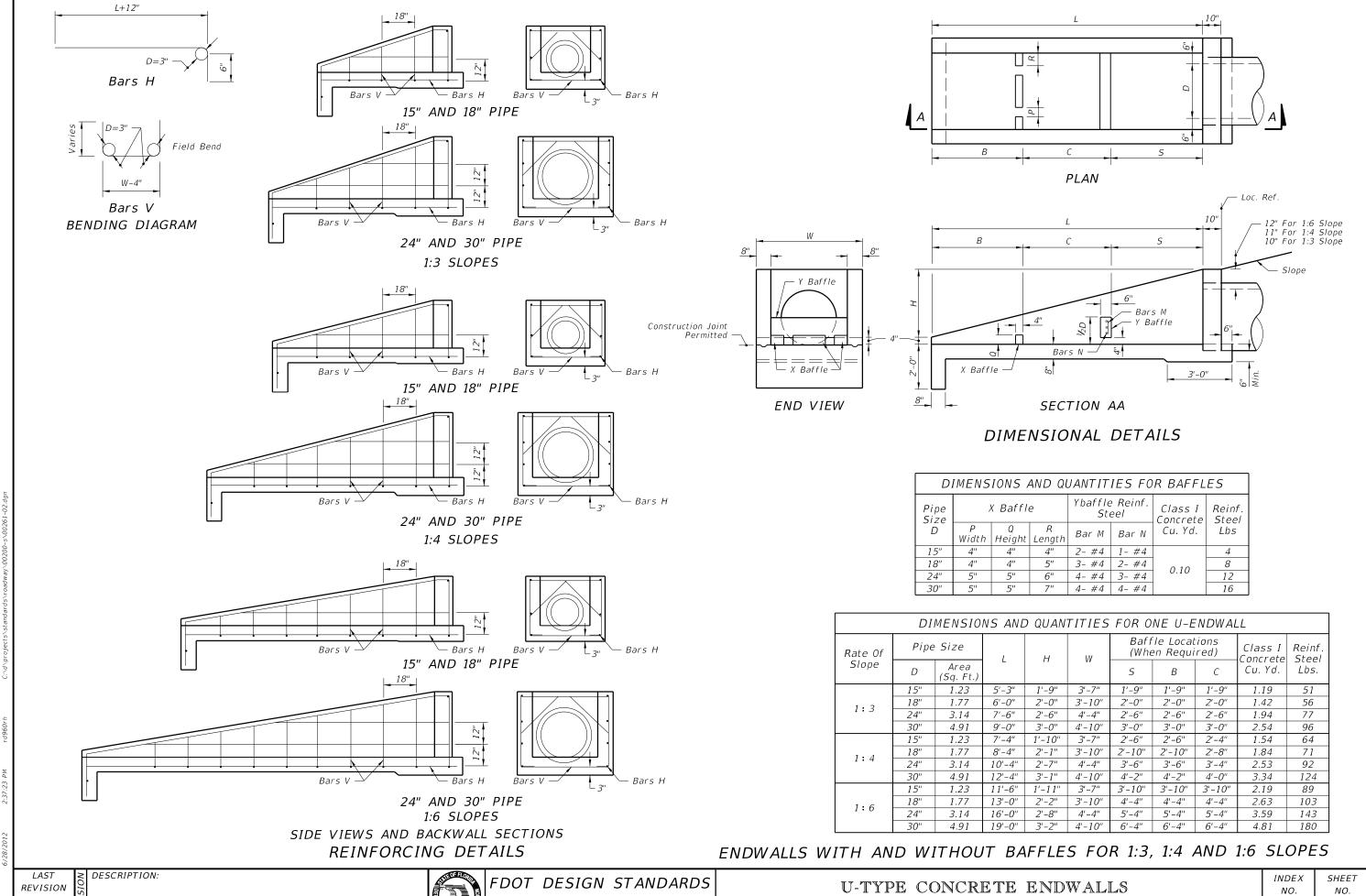


FDOT DESIGN STANDARDS 2013

INDEX SHEET NO. NO.

261

REVISION 07/01/09 U-TYPE CONCRETE ENDWALLS



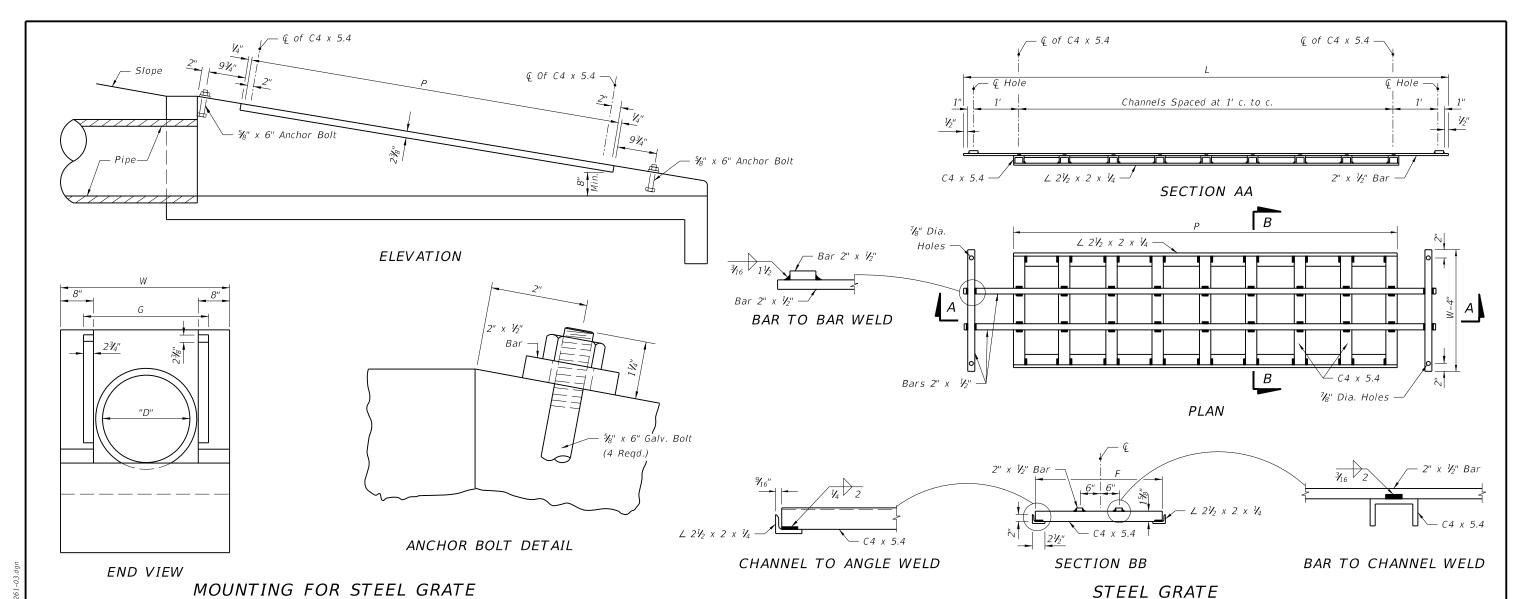
2013

07/01/12

261

BAFFLES & GRATE OPTIONAL 15" TO 30" PIPE

 $\frac{1}{2}$



STEEL GRATING USE CRITERIA

- 1. Grates to be used on pipe culvert endwalls located within the designated clear zone. Positive debris control shall be provided at all upgradient openings. Grates shall not be used unless one or more of the following conditions exist:
- A. Drainage area to culvert consists of median or infield areas or areas where debris and/or drift is negligible.
- B. Runoff to culvert is by sheet flow or in such ill defined channels that debris transport is not considered a major problem.
- C. Runoff to culvert is minor except on an infrequent basis (10 to 15 year frequency); for example a drainage basin in flat sandy terrain with normally low ground water table.
- D. Areas where culvert blockage with resultant backwater would not seriously affect roadway embankment, traffic operation or upland property.
- 2. Steel grating to be used only where called for in plans.

DESCRIPTION:

Rai	I	G	2 E	ach Bars 3.4 lb/ft	@	(X)	Channels 5.4 lb/ft	@	_	gles @ Ib/ft	Total Weight
SIO	"		L	W-4"	lb	(X)	F	Ib	Р	lb	(lb)
	15"	2' -81/2"	9'-3"	3'-3"	85	8	2' -61/8"	111	7'-4"	53	249
1:	6 18"	2' -111/2"	10'-3"	3'-6"	94	9	3' -97/8"	137	8'-4"	62	292
	24"	3' -51/2"	13'-3"	4'-0"	117	12	3' -31/8"	215	11'-4"	82	414
	30"	3' -11½"	16'-3"	4'-6"	141	15	3' -91/8"	310	14'-4"	104	555
	15"	2' -81/2"	6'-3"	3'-3"	65	5	2' -61/8"	70	4'-4"	32	167
1:	4 18"	2' -111/2"	7'-3"	3'-6"	73	6	2' -91/8"	92	5'-4"	39	204
	24"	3' -5½"	9'-3"	4'-0"	90	8	3' -31/8"	144	7'-4"	53	287
	30"	3' -11½"	11'-3"	4'-6"	107	10	3' -91/8"	206	9'-4"	68	381
	15"	2' -81/2"	4'-3"	3'-3"	51	3	2' -61/8"	42	2'-4"	17	110
1::	3 18"	2' -111/2"	5'-3"	3'-6"	60	4	2' -91/8"	61	3'-4"	24	145

3' -31/8"

3' -9⁷/₈"

145

6'-4"

TABLE OF DIMENSIONS AND QUANTITIES FOR ONE GRATE

LAST REVISION 00

FDOT DESIGN STANDARDS 2013

8'-3"

3' -111/2"

4'-0"

4'-6"

87

U-TYPE CONCRETE ENDWALLS BAFFLES & GRATE OPTIONAL 15" TO 30" PIPE

278

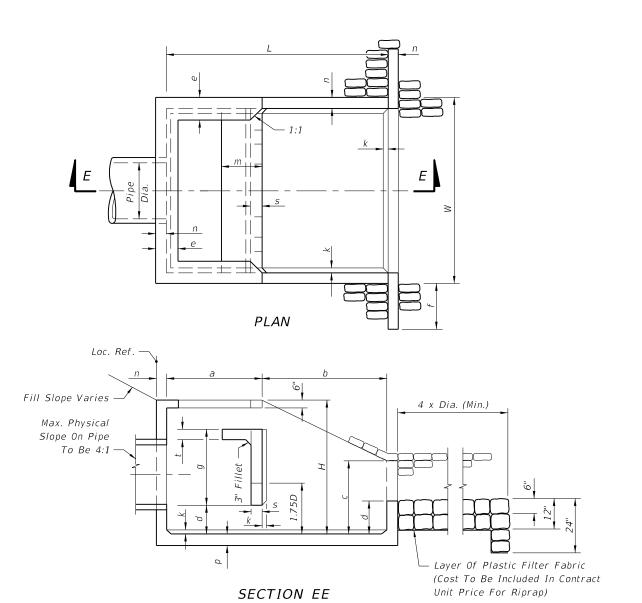
46

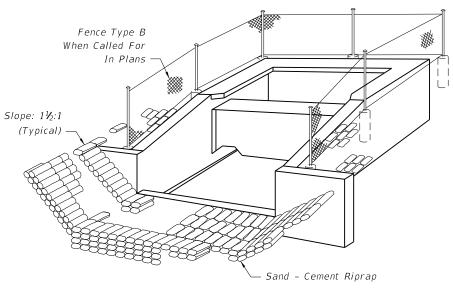
INDEX SHEET NO. NO. 261 3

LAST

REVISION

07/01/09



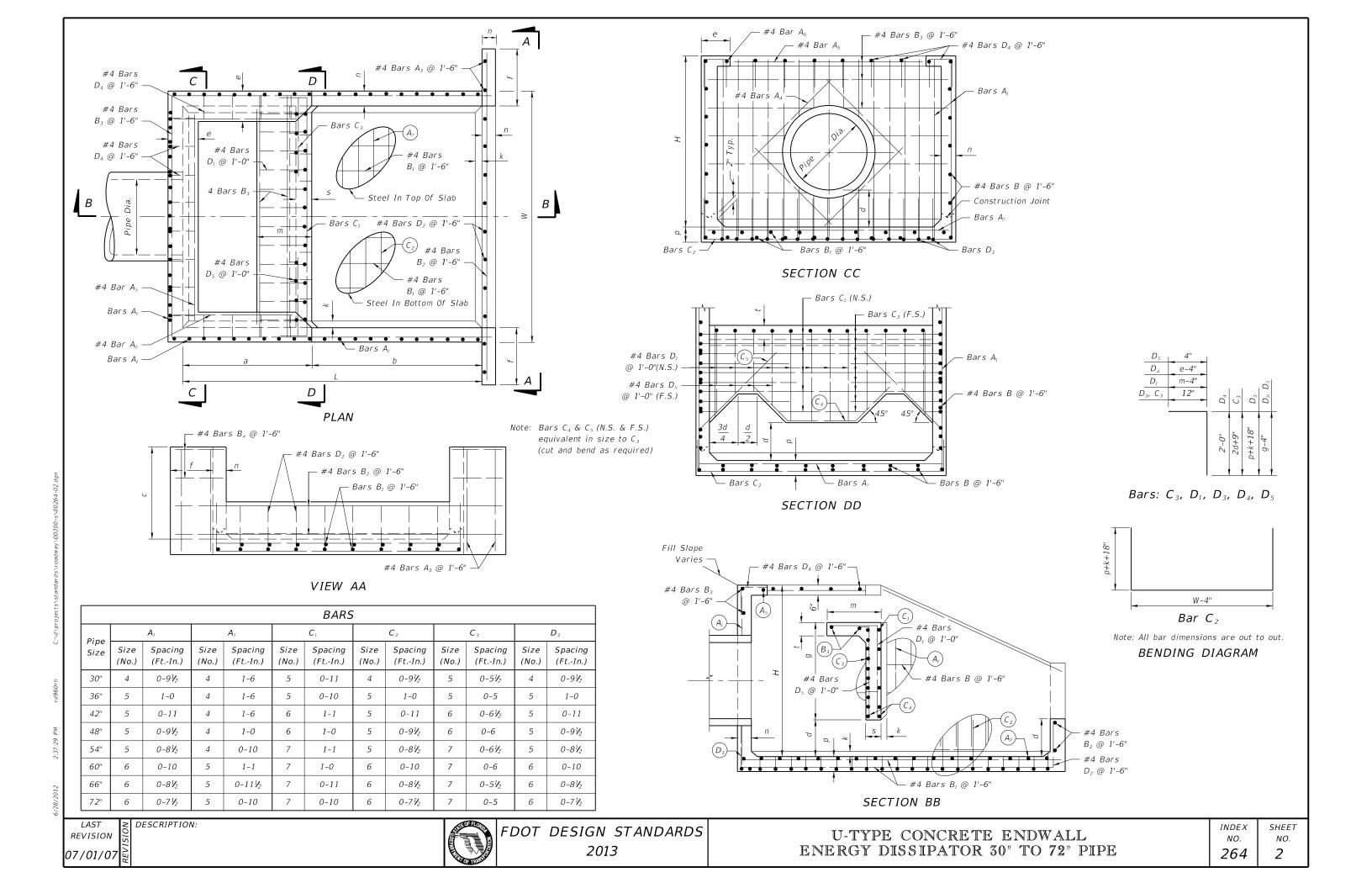


PERSPECTIVE

GENERAL NOTES

- 1. U-type concrete endwall energy dissipators are intended for use outside the clear zone.
- 2. Chamfer all exposed edges ¾".
- 3. Concrete shall be Class I, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 4. Reinforcing steel shall have 2" min. cover.
- 5. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB. Riprap to be paid for under the contract unit price for Riprap (Sand-Cement) (Roadway), CY. Cost of plastic filter fabric to be included in the contract unit price for riprap.
- 6. Fencing, when called for in the plans, to be paid for under the contract unit price for Fencing, Type B, LF. See Index No. 802 for details of Type B fencing.

Pipe	Size	Q							Di	mensio	ns								Concrete	Reinf.	Sand-Cement
Dia.	Area	(Max)					Fee	t - Inc	hes						I	nche.	s		Class I	Steel	Riprap (Nom.)
(in)	(SF)	(cfs)	W	Н	L	а	b	с	d	е	f	g	m	n	р	5	t	k	(CY)	(lb)	(CY)
30	4.91	59	9-0	6-3	10-8	4-7	6-1	3-4	1-4	1-2	2-6	3-0	1-11	6	6½	7	7	3	6.72	736	10.6
36	7.07	85	10-5	7-3	12-4	5-3	7-1	3-10	1-7	1-3	3-0	3-6	2-3	7	71/2	8	8	3	10.34	1,072	13.6
42	9.62	115	11-10	8-0	14-0	6-0	8-0	4-5	1-9	1-6	3-0	3-11	2-6	8	8½	9	8	4	14.82	1,429	17.5
48	12.57	151	13-3	9-0	15-8	6-9	8-11	4-11	2-0	1-7	3-0	4-5	2-10	9	91/2	10	8	4	20.36	2,000	22.1
54	15.90	191	14-8	9-9	17-4	7-4	10-0	5-5	2-2	1-10	3-0	4-11	3-0	10	101/2	10	8	4	27.19	2,659	27.2
60	13.63	236	16-1	10-9	19-0	8-0	11-0	5-11	2-5	1-11	3-0	5-4	3-4	11	111/2	11	8	6	34.49	3,552	32.5
66	23.76	285	17-3	11-6	20-6	8-8	11-10	6-5	2-7	2-1	3-0	5-9	3-7	12	121/2	12	8	6	42.82	4,472	38.3
72	28.27	339	18-6	12-3	22-0	9-3	12-9	6-11	2-9	2-3	3-0	6-2	3-9	12	12½	12	8	6	50.68	5,426	44.5

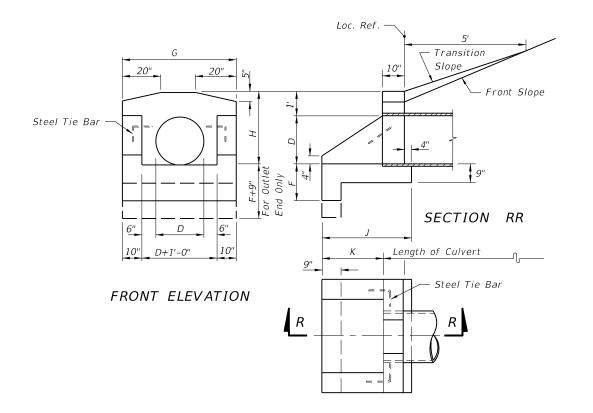


LAST

REVISION

07/01/09





PLANCONCRETE ENDWALL WITH U-TYPE WINGS FOR PIPE CULVERTS

TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH U-TYPE WINGS

			DIMENS.	IONS					QL	JANTITIES	S IN ON	IE ENDW	4 <i>LL</i>
Ор	ening		Wall		Foc	ting		Concr	ete, Cla	ss I, Tot	al (CY)		Steel
D	Area	G	Н	Κ	F	,	R	CP	С	MP	C	CIP	Tie Bars
	(ft²)	6	п		Γ	J	Inlet	Outlet	Inlet	Outlet	Inlet	Outlet	THE Dats
12"	0.8	3'-8"	2'-0"	1'-0"	1'-3"	2'-2"	0.48			0.49	0.57	none	
15"	1.2	3'-11"	2'-3"	1'-5"	1'-3"	2'-7"	0.59	0.67	0.62	0.70	0.61	0.70	none
18"	1.8	4'-2"	2'-6"	1'-9"	1'-3"	2'-11"	0.70	0.79	0.74	0.82	0.74	0.82	none
24"	3.1	4'-8"	3'-0"	2'-6"	1'-6"	3'-8"	1.01	1.11	1.06	1.16	1.06	1.16	2-#6 Bars x 2'-0"
30"	4.9	5'-2"	3'-6"	3'-3"	1'-6"	4'-5"	1.33	1.44	1.41	1.51	1.40	1.51	2-#6 Bars x 2'-0"
36"	7.1	5'-8"	4'-0"	4'-0"	1'-9"	5'-2"	1.73	1.85	1.84	1.96	1.82	1.94	2-#6 Bars x 2'-6"
42"	9.6	6'-2"	4'-6"	4'-9"	2'-0"	5'-11"	2.19	2.32	2.32	2.45			2-#6 Bars x 2'-6"
48"	12.6	6'-8"	5'-0"	5'-6"	2'-0"	6'-8"	2.64	2.78	2.81	2.95			2-#6 Bars x 3'-0"

— Transition Slope Front Slope Steel Tie Bar SECTION NN FRONT ELEVATION Length of Culvert Steel Tie Bar PLAN

CONCRETE ENDWALL WITH 45° WINGS FOR PIPE CULVERTS

TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH 45° WINGS

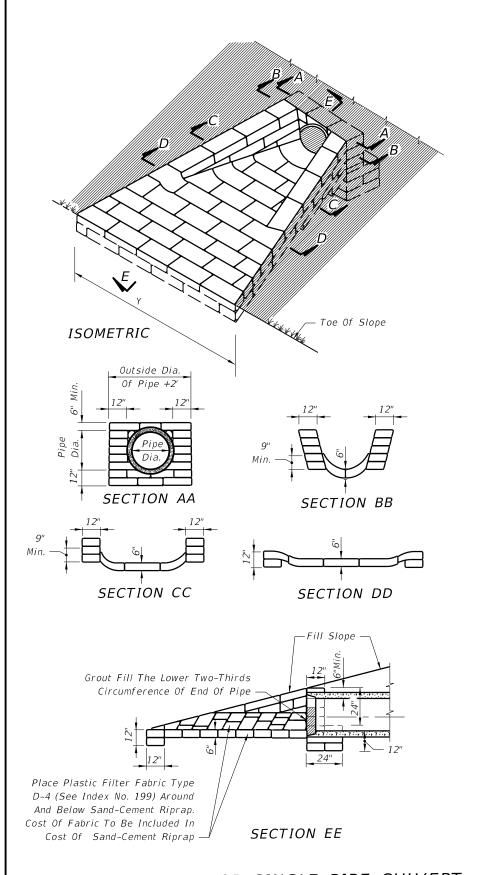
			DIMENSI	ONS				QUANTIT	IES IN O	NE ENDWALL
Op	ening		W	all		Footing	Concre	te, Class	I	
D	Area	Н	G	,	М	F	Tot	al (CY)		Steel Tie Bars
	(ft²)	П	G		IVI	<i></i>	RCP	СМР	CIP	
15"	1.2	2'-3"	3'-7"	1'-0"	1'-3"	1'-3"	0.56	0.59	0.59	none
18"	1.8	2'-6"	3'-10"	1'-2"	1'-7"	1'-3"	0.74	0.77	0.77	none
24"	3.1	3'-0"	4'-4"	1'-5"	2'-1"	1'-4"	1.01	1.06	1.06	2 -#6 Bars x 2'-0"
30"	4.9	3'-6"	4'-10"	1'-9"	2'-5"	1'-6"	1.32	1.40	1.39	2 -#6 Bars x 2'-0"
36"	7.1	4'-0"	5'-4"	2'-0"	2'-11"	1'-8"	1.72	1.83	1.82	2 -#6 Bars x 2'-6"
42"	9.6	4'-6"	5'-10"	2'-3"	3'-6"	2'-0"	2.34	2.47		2 -#6 Bars x 2'-6"
48"	12.6	5'-0"	6'-4"	2'-6"	4'-0"	2'-0"	2.74	2.90		2 -#6 Bars x 2'-6"

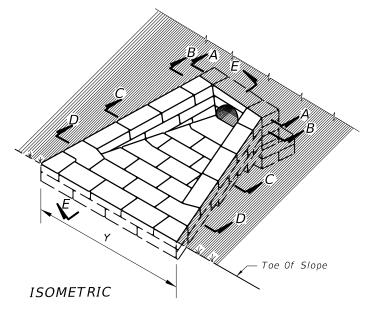
GENERAL NOTES

- 1. Winged concrete endwalls are intended for use outside the clear zone.
- 2. Chamfer all exposed edges ¾".
- 3. Concrete shall be Class I, except ASTM C478 (4000 psi) Concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications.
- 4. Endwall to be paid for under the contract unit price for Class I Concrete.
- 5. Sodding to be in accordance with Index No. 281, and paid for under the contract unit price for Performance Turf, SY.

DESCRIPTION:

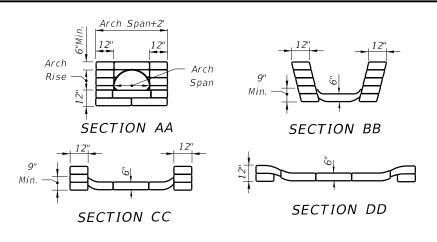
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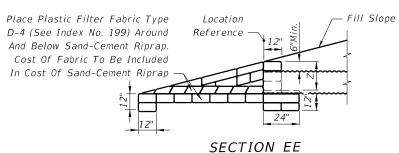




GENERAL NOTES

1. U-Type Sand-Cement Endwalls Are Intended For Use Outside The Clear Zone.





DETAILS FOR SINGLE METAL PIPE ARCH CULVERTS

NOTE: For multiple metal pipe arch culvert spacing between arch centers=X

		5.7	NATNIC	70116 4	ND 01				457 41	0105	40011	GUUV	- DT C		
		DI	MENSI	IONS A	אט ענ	JANII	ILES	FOR N	1E I AL	PIPE .	AKCH	CULV	EKIS		
Span	Rise			Dimen	sions				(Quantity In Cu.	Of Sand Yds. Foi				
Spari	Mise	X		Y	,		7		For 1:2	Slopes			For 1:4	Slopes	
		Λ,	1-Arch	2-Arch	3-Arch	4-Arch		1-Arch	2-Arch	3-Arch	4-Arch	1-Arch	2-Arch	3-Arch	4-Arch
17"	13"	2'-6"	6'-6"	9'-0"	11'-6"	14'-0"	1'-7"	1.0	1.5	2.0	2.5	1.5	2.2	2.9	3.6
21"	15"	2'-10"	6'-6" 9'-0" 11'-6" 14'-0' 7'-6" 10'-4" 13'-2" 16'-0'				1'-9"	1.2	1.8	2.4	3.0	1.9	2.7	3.5	4.3
28"	20"	3'-5"	9'-3"	12'-8"	16'-1"	19'-6"	2'-0"	1.7	2.5	3.3	4.1	2.6	3.7	4.8	5.9
35"	24"	4'-0"	11'-0"	15'-0"	19'-0"	23'-0"	2'-0"	2.2	3.1	4.0	4.9	3.4	4.7	6.0	7.3
42"	29"	4'-9"	12'-9"	17'-6"	22'-3"	27'-0"	2'-0"	2.9	4.1	5.3	6.5	4.5	6.1	7.7	9.3
49"	33"	5'-6"	14'-6"	20'-0"	25'-6"	31'-0"	2'-0"	3.5	4.9	6.3	7.7	5.5	7.4	9.3	11.2
57"	38"	6'-4"	16'-6"	22'-10"	29'-2"	35'-6"	2'-0"	4.4	6.1	7.8	9.5	6.9	9.2	11.5	13.8
64"	43"	7'-1"	18'-3"	25'-4"	32'-5"	39'-6"	2'-0" 5.1 7.0 8.9 10.8 8.1 10.7 13.3 15.9								15.9
71"	47"	7'-10"	20'-0"	27'-10"	35'-8"	43'-6"	2'-0"	5.9	8.1	10.3	12.5	9.5	12.4	15.3	18.2

		DIM	ENSIOI	VS AND	QUAN	ITITIE	S FOR	ROUN	ID PIPI	E CUL	VERTS		
Pipe			Dimensio	ns					y Of San ı. Yds. Fo				
Dia.	V			Υ			For 1:2	Slopes			For 1:4	Slopes	
	X	1-Pipe	2-Pipes	3-Pipes	4-Pipes	1-Pipe	2-Pipes	3-Pipes	4-Pipes	1-Pipe	2-Pipes	3-Pipes	4-Pipes
15"	2'-7"	7'-0"	9'-7"	12'-2"	14'-9"	1.2	1.6	2.1	2.6	1.7	2.4	3.0	3.6
18"	2'-10"	8'-0"	10'-10"	13'-8"	16'-6"	1.4	2.0	2.6	3.1	2.1	2.9	3.7	4.4
24"	3'-5"	10'-0'	13'-5"	16'-10"	20'-3"	1.9	2.7	3.5	4.3	2.9	4.0	5.1	6.3
30"	4'-3"	12'-0"	16'-3"	20'-6"	24'-9"	2.5	3.6	4.8	5.9	3.8	5.4	7.0	8.6
36"	5'-1"	14'-0"	19'-1"	24'-2"	29'-3"	3.1	4.6	6.2	7.7	4.8	7.0	9.2	11.4
42"	6'-0"	16'-0"	22'-0"	28'-0"	34'-0"	3.8	5.8	7.7	9.7	6.0	8.8	11.7	14.5
48"	6'-9"	18'-0"	24'-9"	31'-6"	38'-3"	4.5	7.0	9.4	11.8	7.2	10.8	14.3	17.9
54"	7'-8"	20'-0"	27'-8"	35'-4"	43'-0"	5.3	8.3	11.3	14.2	8.5	12.9	17.3	21.7
60"	8'-6"	22'-0"	30'-6"	39'-0"	47'-6"	6.2	9.7	13.3	16.9	10.0	15.3	20.6	25.9

DETAIL FOR SINGLE PIPE CULVERT

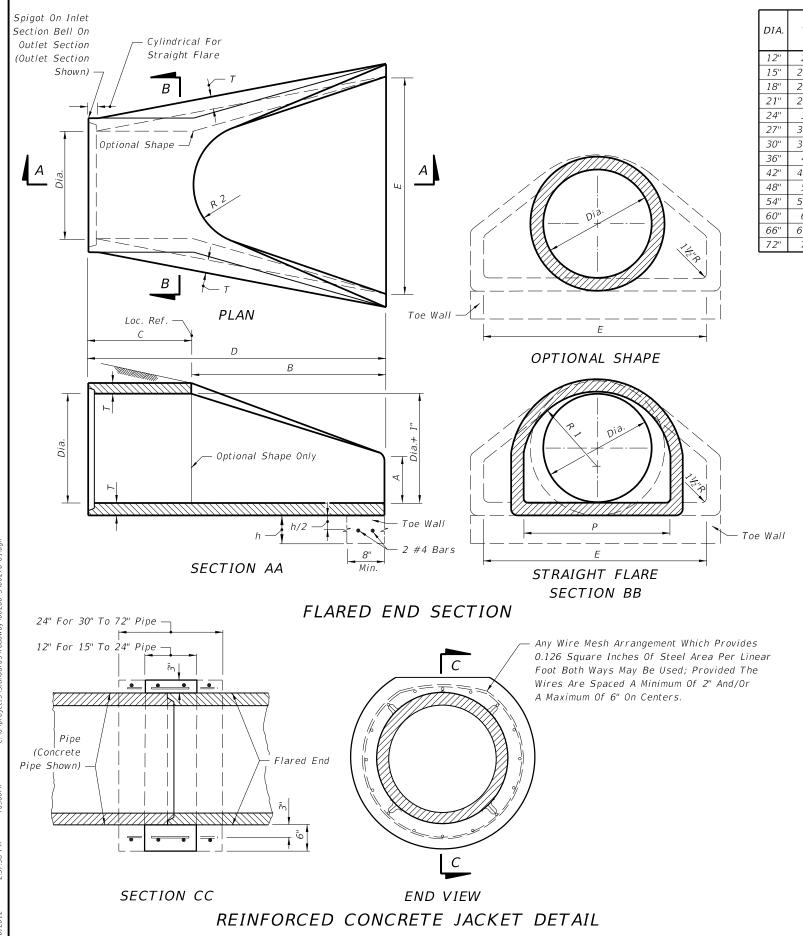
DESCRIPTION:

LAST

REVISION

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Note: For multiple pipe culvert spacing between pipe centers=X



		REINF.	BELL										WEIGHT		TOE WALL
DIA.	T	(in²/ft)	Or	Α	В	С	D	E	Р	R 1	R 2	FLAT	(lb)	h	CLASS I CONC
		(111712)	Spigot										(10)		(CY)
12"	2"	0.07	1½"	4"	2'-0"	4'-0 ⁷ / ₈ ''	6'-0 ⁷ / ₈ ''	2'-0"	19 ¹ ⁄⁄ ₁₆ "	10⅓"	9"	3½"	530	12"	.06
15"	21/4"	0.07	2"	6"	2'-3"	3'-10"	6'-1"	2'-6"	245⁄ ₁₆ "	12½"	11"	3½"	740	12"	.07
18"	21/2"	0.07	21/2"	9"	2'-3"	3'-10"	6'-1"	3'-0"	29"	15½"	12"	4"	990	15"	.11
21"	23/4"	0.07	21/4"	9"	2'-11"	3'-2"	6'-1"	3'-6"	315/8"	16½"	13"	4"	1280	15"	.12
24"	3"	0.07	21/2"	91/2"	3'-7 ¹ / ₂ "	2'-6"	6'-1½"	4'-0"	33¾ ₁₆ "	16 ¹³ / ₁₆ "	14"	4½"	1520	18"	.17
27"	31/4"	0.148	21/2"	10½"	4'-0"	2'-11/2"	6'-1½"	4'-6"	36"	18% ₁₆ "	14½"	4½"	1930	18"	.19
30"	31/2"	0.148	3"	1'-0"	4'-6"	1'-7¾"	6'-1¾''	5'-0"	37"	18½"	15"	5"	2190	21"	.24
36"	4"	0.148	3½"	1'-3"	5'-3"	2'-10¾"	8'-1¾''	6'-0"	47 ¹ 3⁄ ₁₆ "	24 ⁵ ⁄ ₁₆ "	20"	5½"	4100	21"	.29
42"	4½"	0.148	3¾"	1'-9"	5'-3"	2'-11"	8'-2"	6'-6"	53 ⁷ /8"	27½"	22"	5½"	5380	24"	.36
48"	5"	0.148	4½"	2'-0"	6'-0"	2'-2"	8'-2"	7'-0"	56½"	28½"	22"	5¾"	6550	24"	.39
54"	5½"	0.174	4¾"	2'-3"	5'-5"	2'-11"	8'-4"	7'-6"	65½"	331⁄8"	24"	6½"	8040	24"	.42
60"	6"	0.174	5"	2'-6"	5'-0"	3'-3"	8'-3"	8'-0"	72½"	36 ¹ 1 / ₁₆ "	24"	6¾"	8750	24"	.44
66"	6½"	0.174	5½"	2'-0"	6'-6"	1'-9"	8'-3"	8'-6"	72"	36½"	24"	71/4"	10630	24"	.47
72"	7"	0.174	6"	2'-0"	6'-6"	1'-9"	8'-3"	9'-0"	77 ¹³ / ₁₆ "	38 ¹⁵ ⁄ ₁₆ "	24"	73/4"	12520	24"	.50

GENERAL NOTES

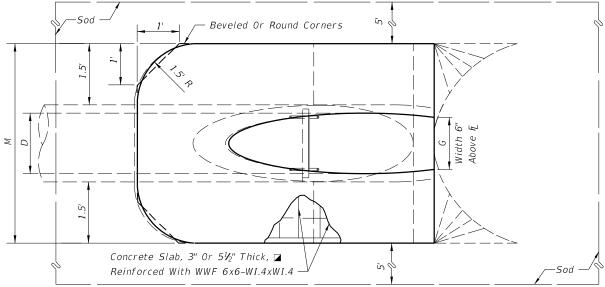
- 1. Flared end sections shall conform to the requirements of ASTM C76 with the exception that dimensions and reinforcement shall be as prescribed in the table above. Circumferential reinforcement may consist of either one cage or two cages of steel. Compressive strength of concrete shall be 4000 psi. Shop drawings for flared end sections having dimensions other than above must be submitted for approval to the State Drainage Engineer.
- 2. Connections between the flared end section and the pipe culvert may be any of the following types unless otherwise shown on the plans.
- a. Joints meeting the requirements of Section 449 of the Standard Specifications (O-Ring Gasket). Flared end section joint dimensions and tolerances shall be identical or compatible to those used in the pipe culvert joint. When pipe culvert and flared end section manufacturers are different, the compatibility of joint designs shall be certified to by the manufacturer of the flared end sections.
- b. Joints sealed with preformed plastic gaskets. The gaskets shall meet the requirements of Section 942-2 of the Standard Specifications and the minimum sizes for gaskets shall be as that specified for equivalent sizes of elliptical pipe.
- c. Reinforced concrete jackets, as detailed on this drawing. Cost of the reinforced concrete jacket to be included in the contract unit price for the flared end section. When non-coated corrugated metal pipe is called for in the plans, the pipe shall be bituminous coated in the jacketed area as specified on Index No. 280. Bituminous coating to be included in the contract unit price for the pipe culvert. Concrete jacket shall be as specified on Index No. 280. Cost of concrete and reinforcement shall be included in the contract unit price for the pipe culvert.
- 3. Toe walls shall be constructed when shown on the plans or at locations designated by the Engineer. Toe walls are to be cast-in-place with Class I Concrete and paid for under the contract unit price for Flared End Section (Concrete), EA. Reinforcing steel shall also be included in the cost of the Flared End Section (Concrete), EA.
- 4. On skewed pipe culverts the flared end sections shall be placed in line with the pipe culvert. Side slopes shall be warped as required to fit the flared end sections.
- 5. Flared End Section to be paid for under the contract unit price for Flared End Section (Concrete), EA. Sodding shall be in accordance with Index No. 281, and paid for under the contract unit price for Performance Turf, SY.

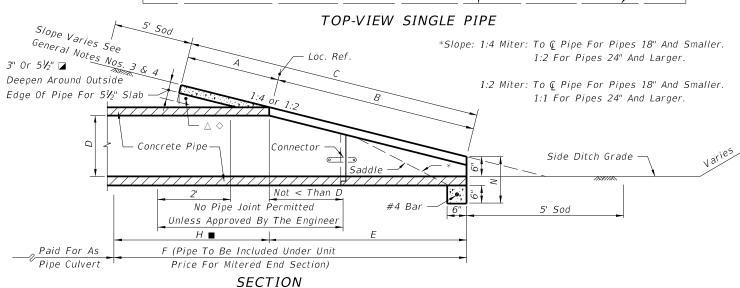
DESIGN NOTES

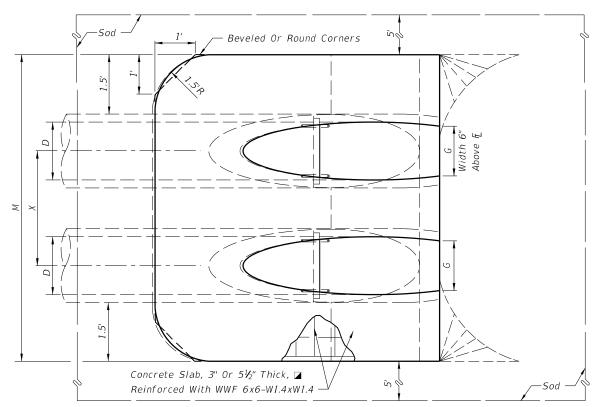
- 1. Flared end sections are intended for use outside the clear zone on median drain and cross drain installation, except that flared end sections for pipe sizes 12" and 15" are permitted within the clear zone. When the slope intersection permits, 12" and 15" flared end sections may be located with the culvert opening as close as 8' beyond the outside edge of the shoulder. Flared end sections are not intended for side drain installations.
- 2. Reinforced concrete jackets shall be used at all locations where high velocities and/or highly erosive soils may cause disjointing. These locations are to be shown on the plans.
- 3. Toe walls shall be used whenever the anticipated velocity of discharge and soil type are such that erosive action would occur. Toe walls are not required where ditch pavement is provided, except when disjointing would occur if the ditch pavement should fail.

DESCRIPTION: LAST REVISION 07/01/09









TOP-VIEW MULTIPLE PIPE

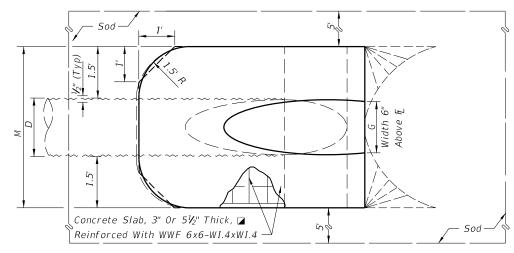
NOTE: See sheet 6 for details and notes.

SINGLE AND MULTIPLE ROUND CONCRETE PIPE

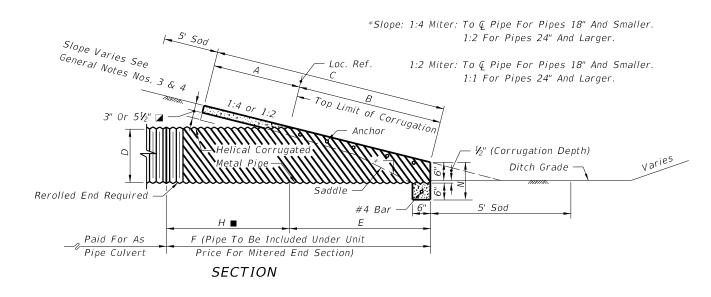
LAST DESCRIPTION:
REVISION USING DESCRIPTION:

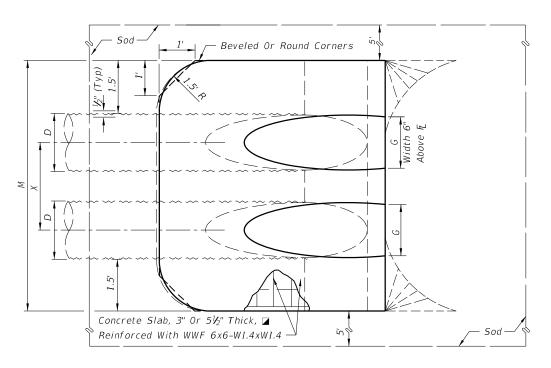


FDOT DESIGN STANDARDS
2013



TOP VIEW-SINGLE PIPE





TOP VIEW-MULTIPLE PIPE

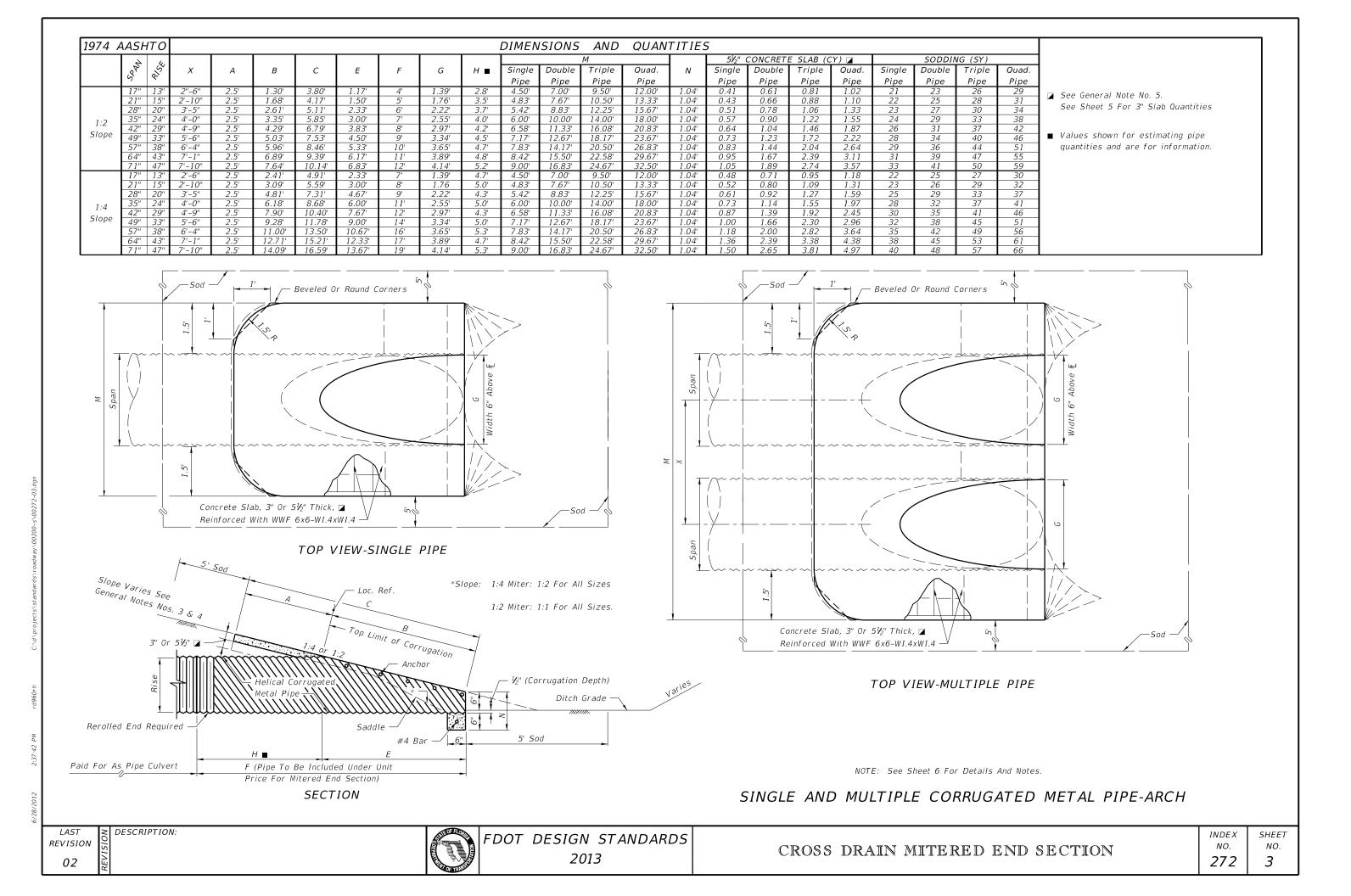
NOTE: See Sheet 6 For Details And Notes.

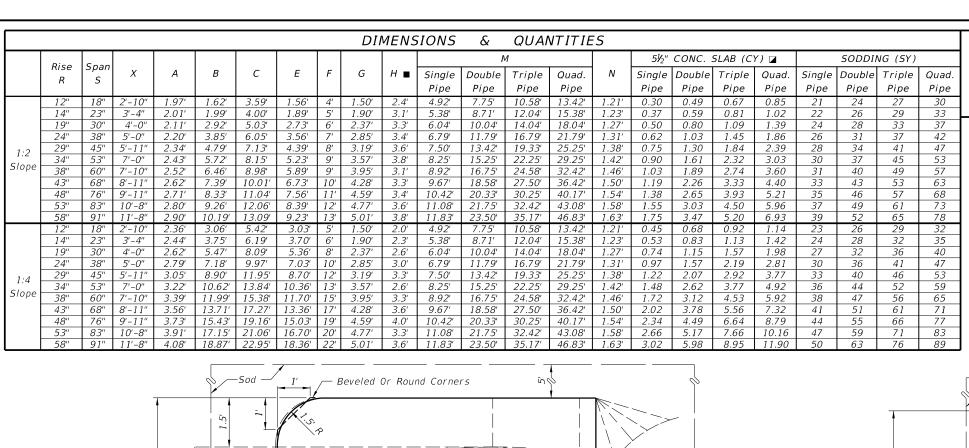
SINGLE AND MULTIPLE ROUND CORRUGATED METAL PIPE

LAST DESCRIPTION:
REVISION 15/1

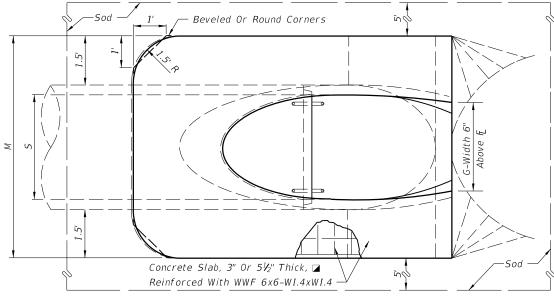


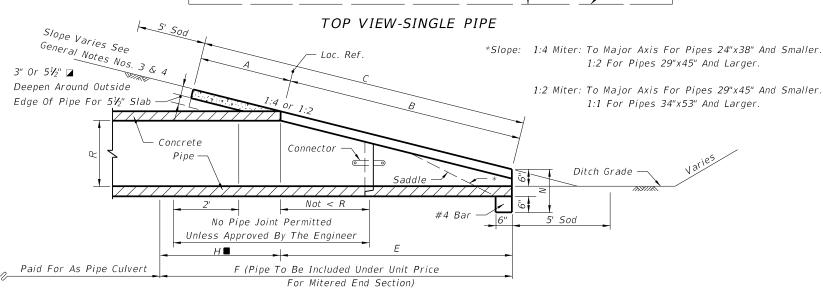
FDOT DESIGN STANDARDS
2013



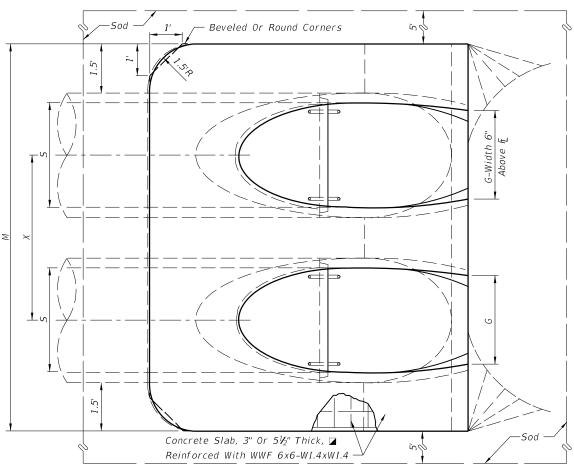


- See General Note No. 3. See Sheet 5 For 3" Slab Quantities
- Values shown for estimating pipe quantities and are for information only.





SECTION



NOTE: See Sheet 6 For Details And Notes.

TOP VIEW - MULTIPLE PIPE

SINGLE AND MULTIPLE ELLIPTICAL CONCRETE PIPE

LAST REVISION 02

DESCRIPTION:

FDOT DESIGN STANDARDS 2013

SHEET INDEXNO. NO. 272 4

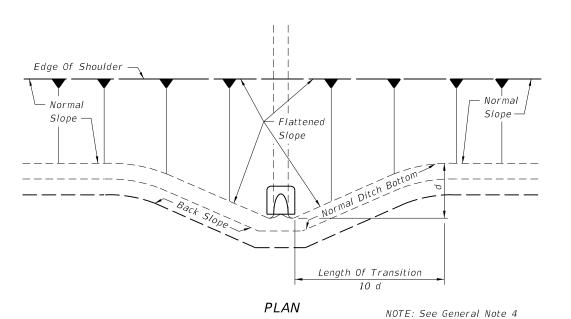
			ROUNI	D-CMP	
	D	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
1:2 Slope	15" 18" 24" 30" 36" 42" 48" 54"	0.24 0.26 0.32 0.38 0.44 0.51 0.57 0.65	0.37 0.43 0.52 0.64 0.78 0.96 1.09 1.32 1.49	0.51 0.61 0.72 0.91 1.13 1.41 1.63 1.99 2.28	0.64 0.78 0.91 1.18 1.48 1.87 2.15 2.66 3.07
1:4 Slope	15" 18" 24" 30" 36" 42" 48" 54" 60"	0.31 0.34 0.44 0.53 0.62 0.71 0.80 0.91 1.02	0.47 0.53 0.69 0.88 1.07 1.30 1.54 1.83 2.15	0.63 0.71 0.92 1.25 1.53 1.92 2.29 2.74 3.27	0.79 0.90 1.18 1.60 2.00 2.52 3.02 3.67 4.39

	u	n)		CMP-ARCH									
	Span	Rise	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe							
	17"	13"	0.33	0.49	0.65	0.81							
	21"	15"	0.33	0.50	0.67	0.83							
	28"	20"	0.37	0.56	0.76	0.95							
	35"	24"	0.40	0.62	0.84	1.07							
	42"	29"	0.43	0.70	0.98	1.25							
1:2	49"	33"	0.49	0.82	1.15	1.48							
Slope	57"	38"	0.55	0.95	1.35	1.75							
	64"	43"	0.62	1.10	1.57	2.05							
	71"	47"	0.69	1.24	1.80	2.35							
	17"	13"	0.38	0.56	0.74	0.92							
	21"	15"	0.39	0.59	0.80	0.95							
	28"	20"	0.43	0.64	0.88	1.10							
	35"	24"	0.49	0.77	1.05	1.33							
١.,	42"	29"	0.57	0.92	1.27	1.62							
1:4	49"	33"	0.65	1.08	1.50	1.93							
Slope	57"	38"	0.76	1.30	1.83	2.37							
	64"	43"	0.87	1.55	2.18	2.83							
	71"	47"	0.95	1.68	2.43	3.17							
					·								

	۵,	_	ELLI	PTICAL-	-CONCR	RETE	
	Rise	Span	Single	Double	Triple	Quad.	
	_	S	Pipe	Pipe	Pipe	Pipe	
	12"	18"	0.19	0.33	0.45	0.57	
	14"	23"	0.25	0.40	0.55	0.69	
	19"	30"	0.34	0.55	0.75	0.95	
	24"	38"	0.43	0.71	1.00	1.28	
	29"	45"	0.52	0.90	1.27	1.65	
1:2	34"	53"	0.62	1.11	1.60	2.09	
Slope	38"	60"	0.70	1.29	1.87	2.46	
	43"	68"	0.81	1.54	2.26	2.99	
	48"	76"	0.93	1.79	2.66	3.53	
	53"	83"	1.04	2.04	3.03	4.02	
	58"	91"	1.17	2.33	3.49	4.66	
	12"	18"	0.30	0.45	0.61	0.76	
	14"	23"	0.36	0.56	0.76	0.95	
	19"	30"	0.51	0.79	1.08	1.36	
	24"	38"	0.68	1.10	1.53	1.96	
	29"	45"	0.86	1.45	2.04	2.63	
1:4	34"	53"	1.02	1.81	2.60	3.39	
Slope	38"	60"	1.18	2.14	3.10	4.05	
	43"	68"	1.38	2.58	3.79	4.99	
	48"	76"	1.59	3.05	4.51	5.97	
	53"	83"	1.80	3.50	5.19	6.88	
	58"	91"	2.04	4.04	6.05	8.05	
				ı	ı	ı	

REVISION

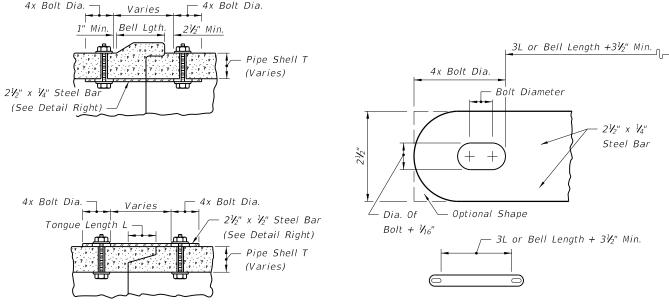




SLOPE AND DITCH TRANSITIONS

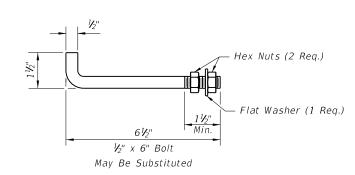
GENERAL NOTES

- 1. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of cross drain pipe; corrugated steel pipe mitered end sections may be used with any type of cross drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of cross drain pipe except steel pipe. When bituminous coated metal pipe is specified for cross drain pipe, mitered end sections shall be constructed with like pipe or concrete pipe. When the mitered end section pipe is dissimilar to the cross drain pipe, a concrete jacket shall be constructed in accordance with Standard Index 280.
- 2. Corrugated polyethylene pipe (HDPE), polyvinyl-chloride pipe (PVC) and polypropylene pipe (PPP) for cross drain applications shall utilize either corrugated metal or concrete mitered end sections (MES). When used in conjunction with corrugated (MES), connection shall be by either a formed metal band specifically designated to join HDPE or PVC pipe, with metal pipe or other coupler approved by the State Drainage Engineer. When used in conjunction with a concrete (MES), connection shall be by concrete jacket constructed in accordance with Index No. 280.
- 3. Mitered end sections for pipe sizes 15", 18" and 24" round or equivalent pipe arch or elliptical pipe are permitted within the clear zone. When the slope intersection permits, the mitered end section may be located with the culvert opening as close as 8' beyond the outside edge of the shoulder.
- 4. Slope and ditch transitions shall be used when the normal roadway slope must be flattened to place end section outside clear zone. See detail left.
- 5. The reinforced concrete slab shall be constructed for all sizes of cross drain pipe and cast in place with Class NS concrete. Slabs shall be 5½" thick unless 3" thickness called for in plans.
- 6. Concrete pipe used in the assembly of mitered end sections shall be selective lengths to avoid excessive connections.
- 7. Corrugated metal pipe galvanizing that is damaged during beveling and perforating for mitered end section shall be repaired.
- 8. That portion of corrugated metal pipe in direct contact with the concrete slab and extending 12" beyond shall be bituminous coated prior to placing of the concrete.
- 9. When existing multiple cross drain pipes are spaced other than the dimensions shown in this detail, or have non-parallel axes, or have non-uniform sections, the mitered end sections will be constructed either separately as single pipe mitered end sections or collectively as multiple pipe end sections as directed by the Engineer; however, mitered end sections will be paid for each based on each independent pipe end.
- 10. The cost of all pipe(s), fasteners, reinforcing, connectors, anchors, concrete, sealants, jackets, and coupling bands shall be included in the cost for the mitered end section. Sodding shall be paid for separately under the contract unit price of Performance Turf, SY.
- 11. Mitered end sections shall be paid for under the contract unit price for Mitered End Section (CD), Each, based on each independent pipe end.



All bars, bolts, nuts and washers are to be galvanized steel. Bolt diameters shall be $\frac{3}{6}$ " for 15" to 36" pipe and $\frac{3}{6}$ " for 42" to 72" pipe. Two connectors required per joint, located 60° right and left of bottom center of pipe. Bolt holes in pipe shell are to be drilled.

CONCRETE PIPE CONNECTOR



Anchors required for CMP only.

Anchor, washer and nuts to be galvanized steel.

Bend anchor where required to center in concrete slab. Damaged surfaces to be repaired after bending. Anchors are to be spaced a distance equal to four (4) corrugations. Place the anchors in the outside crest of corrugation.

Flat washers to be placed on inside wall of pipe.

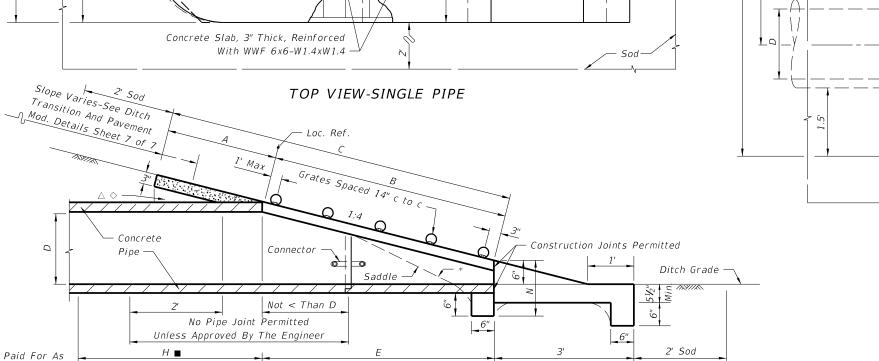
Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

ANCHOR DETAIL

SPECIAL DETAILS AND NOTES

LAST REVISION 07/01/12





Fastener

F (Pipe To Be Included Under Unit Price For Mitered End Section)

SECTION

Sod Permitted

Construction Joints

Permitted

Grate

Grate

Grate

Grate

Grate

Grate

Grate

Fastener

Concrete Slab, 3' Thick, Reinforced

With WWF 6x6-W1.4xW1.4

TOP VIEW-MULTIPLE PIPE

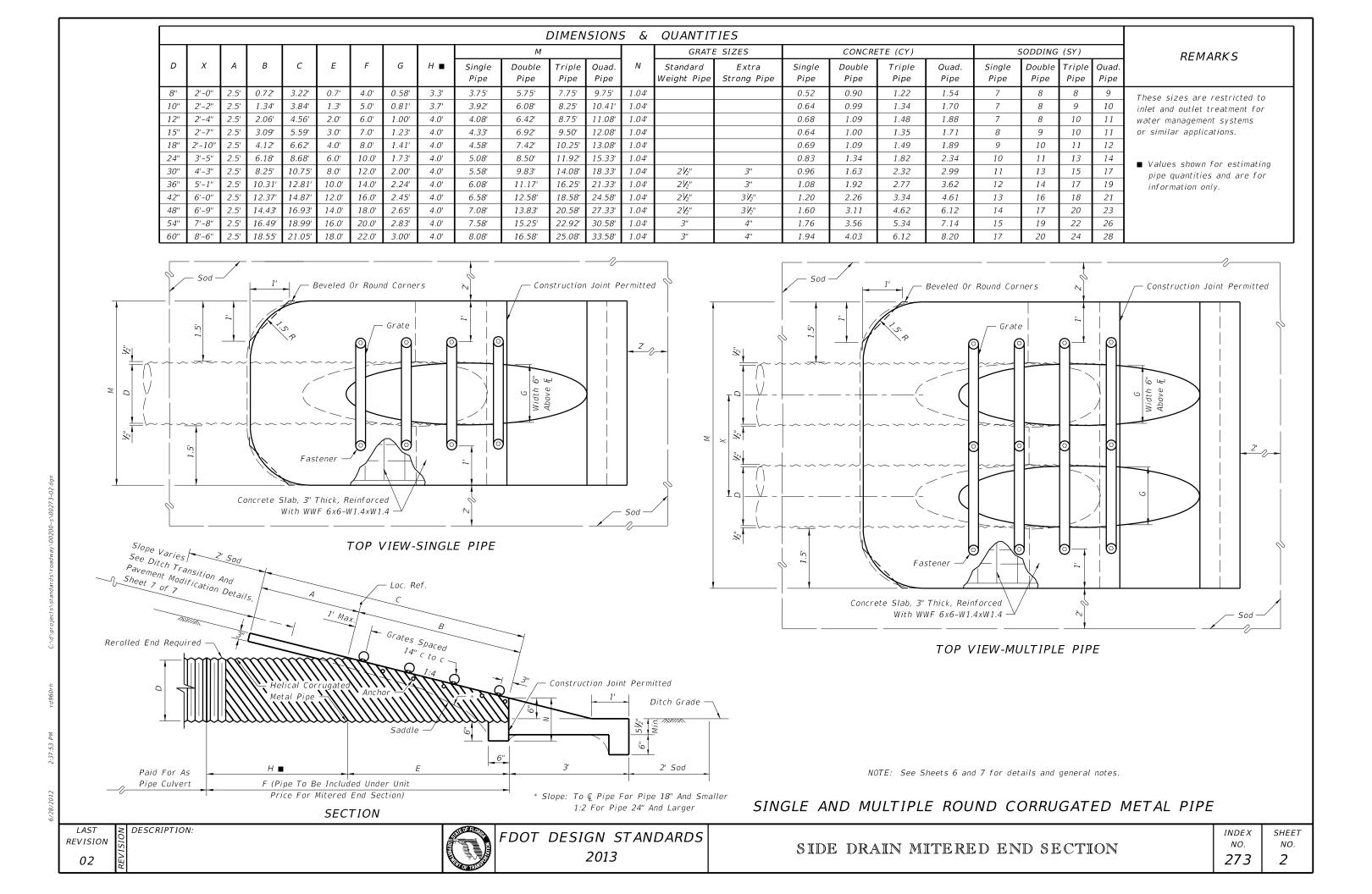
Note: See Sheets 6 and 7 for details and general notes.

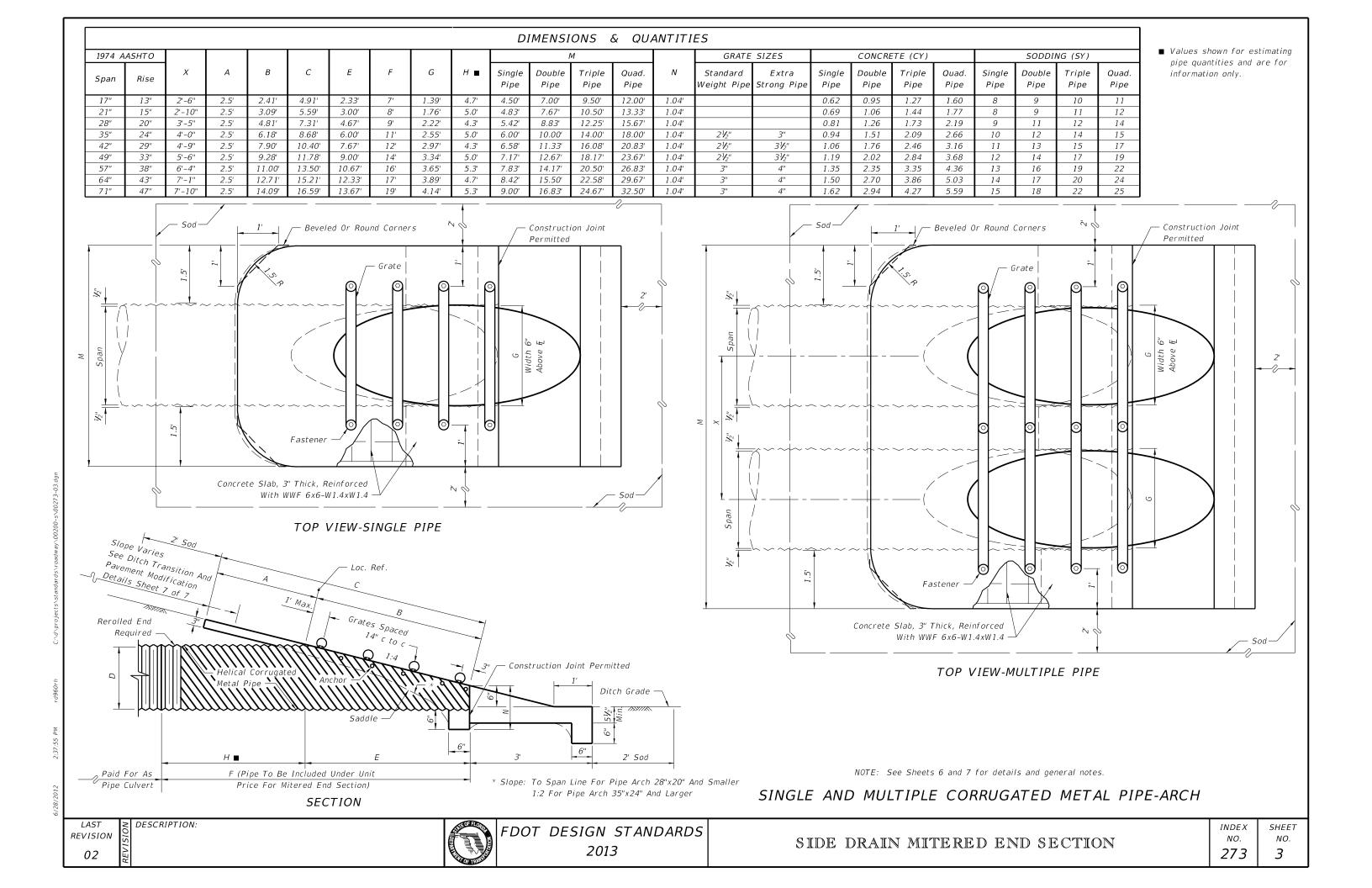
SINGLE AND MULTIPLE ROUND CONCRETE PIPE

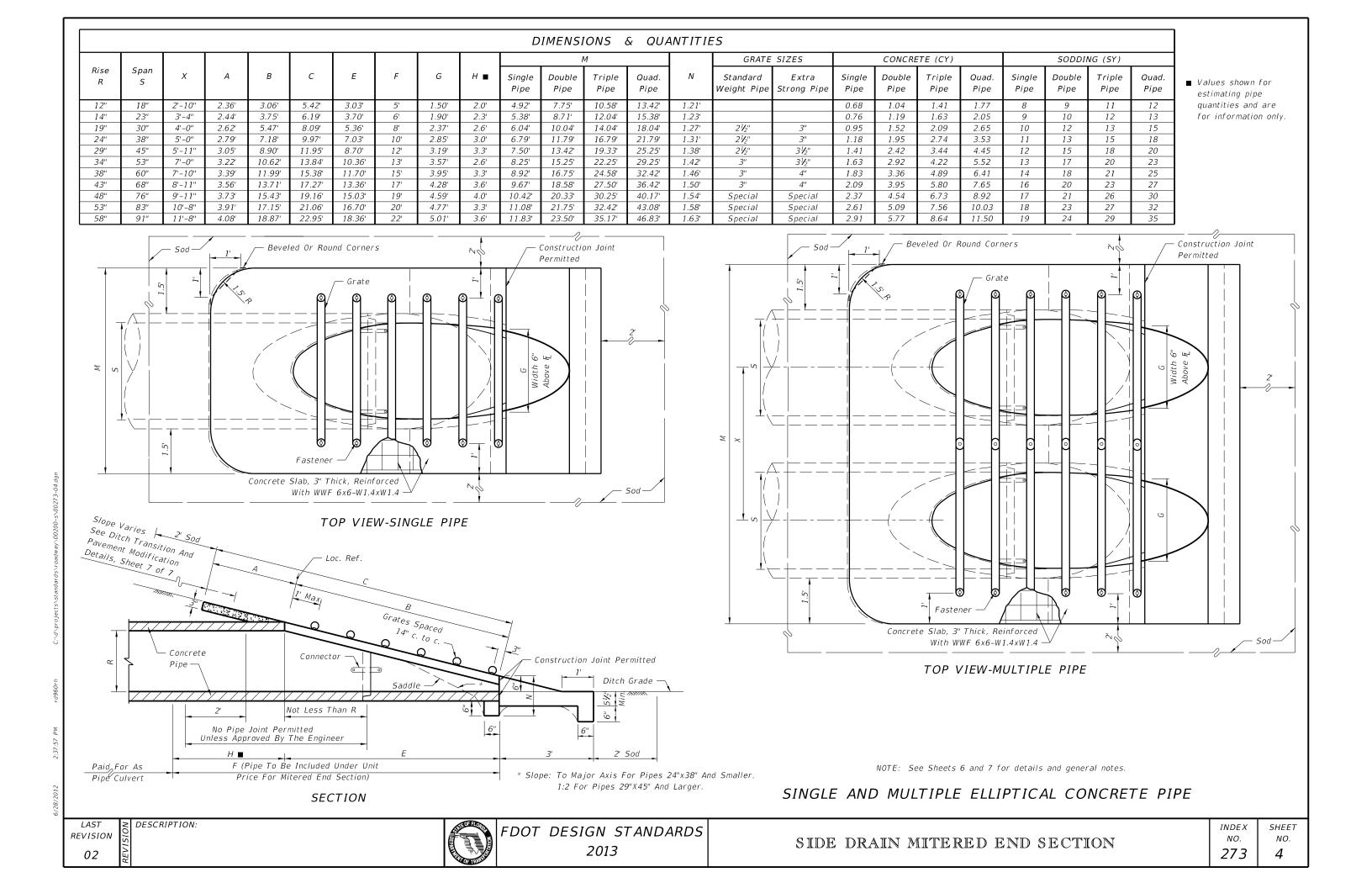
LAST NO DESCRIPTION:

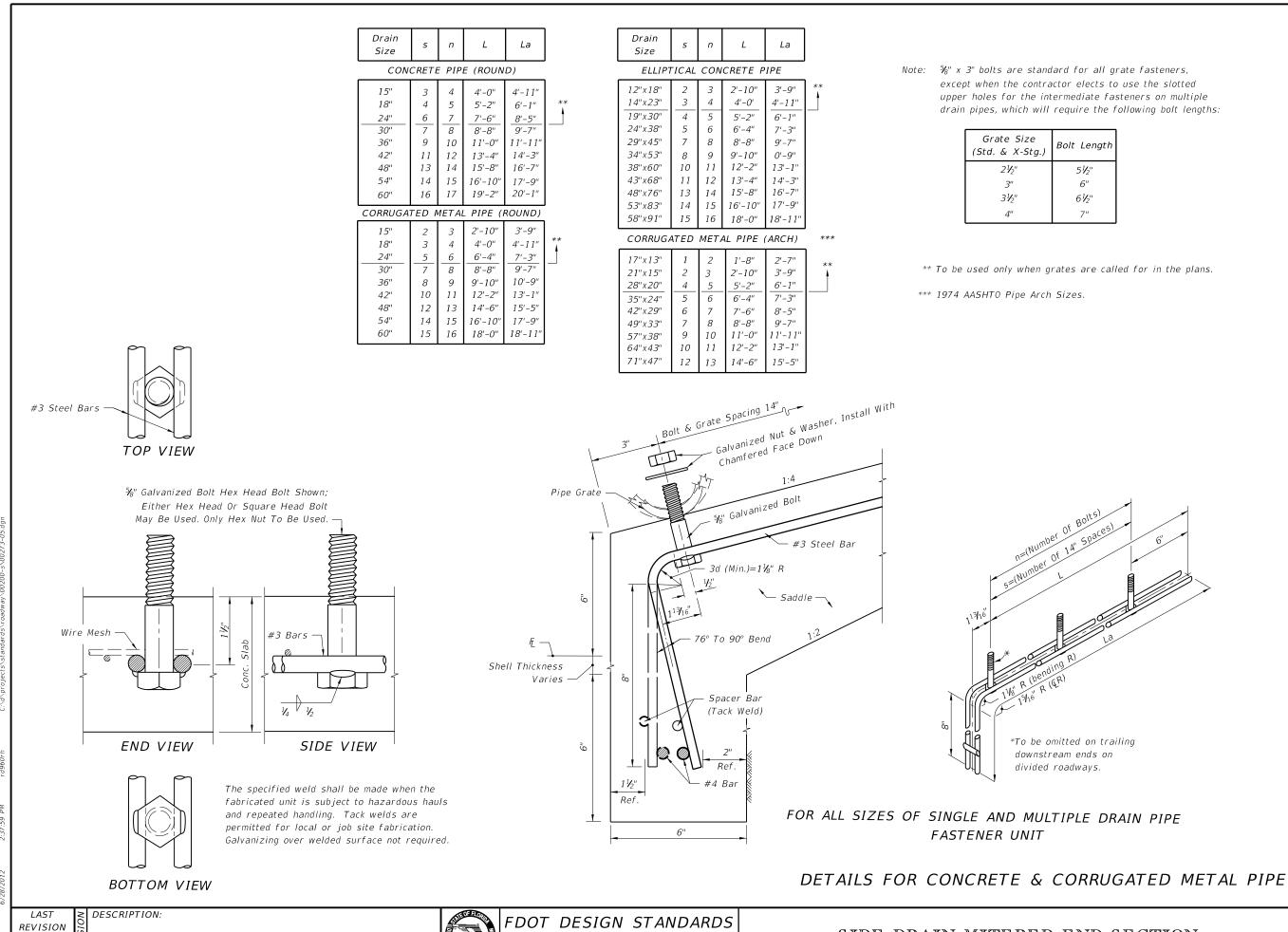


*Slope: To © Pipe For Pipes 18" And Smaller 1:2 For Pipes 24" And Larger.









2013

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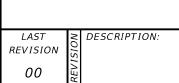
NO. SHEET NO. NO. **273 5**

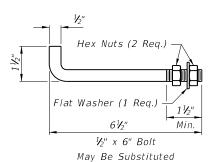












Notes:

Anchors required for CMP only.

Anchor, washer and nuts to be galvanized steel.

Bend anchor where required to center in concrete slab.

Damaged surfaces to be repaired after bending.

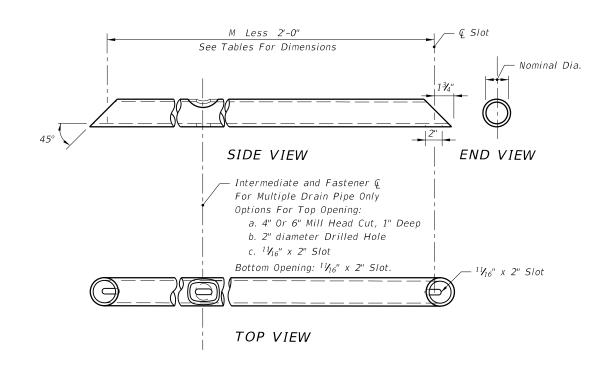
Anchors are to be spaced a distance equal to four (4) corrugations.

Place the anchors in the outside crest of corrugation.

Flat washer to be placed on inside wall of pipe.

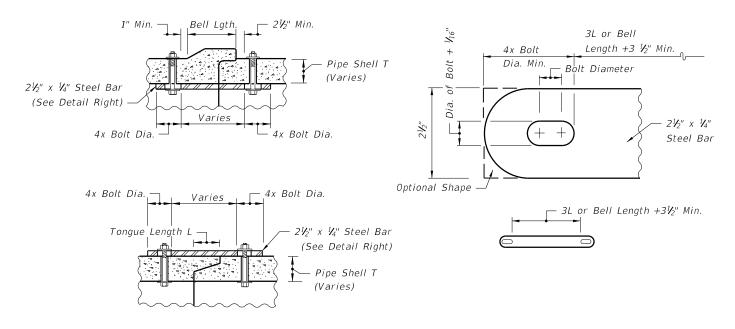
Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

ANCHOR DETAIL



FOR SINGLE & MULTIPLE DRAIN PIPE GRATE DETAIL

See General Notes, Sheet 7.



All bars, bolts, nuts and washers are to be galvanized steel.

Bolt diameters shall be $\frac{3}{6}$ " for 15" to 36" pipe and $\frac{5}{6}$ " for 42" to 60" pipe.

Two connectors required per joint, located 60° right and left of bottom center of pipe.

Bolt holes in pipe shell are to be drilled.

CONCRETE PIPE CONNECTOR DETAIL

DETAILS FOR CONCRETE & CORRUGATED METAL PIPE

A	FDOT	DESIGN	STANDARDS 3
		201.	3

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Intersect 1:4 Slope

And Crown Line Elev.

Modified Slope When Minimum

Cover Or Less Occurs Both On Existing And Proposed Installations

DESCRIPTION:

1:12 Or Steeper



Ditch

Bottom

Flow

Transition Length=10 D

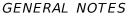
PLAN

DITCH TRANSITION

PERMISSIBLE PAVEMENT MODIFICATION

—Side

Ditch



- 1. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of side drain pipe; corrugated steel pipe mitered end sections may be used with any type of side drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of side drain pipe except steel pipe. When bituminous coated metal pipe is specified for side drain pipe, mitered end sections shall be constructed with like pipe or concrete pipe. When the mitered end section pipe is dissimilar to the side drain pipe, a concrete jacket shall be constructed in accordance with Index No. 280
- 2. Corrugated polyethylene pipe (HDPE), polyvinyl-chloride pipe (PVC) and polypropylene pipe (PPP) for side drain applications shall utilize either corrugated metal or concrete mitered end sections (MES). When used in conjunction with corrugated (MES), connection shall be by either a formed metal band specifically designated to join HDPE or PVC pipe, with metal pipe or other coupler approved by the State Drainage Engineer. When used in conjunction with a concrete (MES), connection shall be by concrete jacket constructed in accordance with Index No. 280.
- 3. Concrete pipe used in the assembly of mitered end sections shall be of selective lengths to avoid excessive connections.
- 4. Corrugated metal pipe galvanizing that is damaged during beveling and perforating for mitered end section shall be repaired.
- 5. That portion of corrugated metal pipe in direct contact with the concrete slab and extending 12" beyond shall be bituminous coated prior to placing of the concrete.
- 6. When existing multiple side drain pipes are spaced other than the dimensions shown in this detail, or have nonparallel axes, or have non-uniform sections, The mitered end sections will be constructed either separately as single pipe mitered end sections or collectively as multiple pipe end sections as directed by the Engineer; however, mitered end sections will be paid for each, based on each independent pipe end.
- 7. In addition to the requirements of Section 430-4, side drain culverts shall comply with the cover requirements shown on Index
- 8. The reinforced concrete slab shall be constructed for all sizes of side drain pipe and cast in place with Class NS concrete.
- 9. Round pipe size 30" or greater, pipe-arch size 35"x24" or greater and elliptical pipe 19"x30" or greater shall be grated unless excepted in the plans. Smaller sizes of pipe shall be grated only when called for in plans. The lower grate on trailing downstream ends on divided highways shall be omitted.
- 10. Grates are to be fabricated from steel ASTM A53, Grade B, pipe. The lower grate on all traffic approach ends shall be Schedule 80 and all remaining grates shall be Schedule 40. Grates subject to salt free and corrosive free environment may be fabricated from galvanized pipe, with base metal exposed during fabrication repaired as specified in Section 562, Standard Specifications; or, fabricated from black pipe and hot dip galvanized after fabrication in accordance with ASTM A123. Grates subject to salt water or highly corrosive environment shall be hot dip galvanized after fabrication in accordance with ASTM A123.
- 11. Ditch transitions shall be used on all grades in excess of 3% as directed by the Engineer.
- 12. The project engineer shall contact the District Drainage Engineer for possible alternate treatment prior to constructing side drain mitered end sections where a minimum spacing of 30' will not result between the toe points of the mitered end sections.
- 13. The cost of all pipe(s), grates, fasteners, reinforcing, connectors, anchors, concrete, sealants, jackets and coupling bands shall be included in the cost for the mitered end section. Sodding shall be paid for separately under the contract unit price for Performance Turf, SY.
- 14. Mitered end sections shall be paid for under the contract unit price for Mitered End Section (SD), Ea., based on each independent pipe end.

DESIGN NOTES

- 1. In critical hydraulic locations, grates shall not be used until potential debris transport has been evaluated by the drainage engineer and appropriate adjustments made. Ditch grades in excess of 3% or pipe with less than 1.5' of cover and grades in excess of 1% will require such an evaluation (General Note 9).
- 2. The design engineer shall determine highly corrosive locations and specify in the plans when the grates shall be hot-dip galvanized after fabrication (General Note 10).
- 3. The design engineer shall determine and designate in the plans which alternate types of mitered end section will not be permitted. The restriction shall be based on corrosive or structural requirements.

NOTES & INFORMATION

FDOT DESIGN STANDARDS



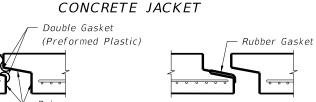
Classes II,III,IV,V; Wall A,B,C									
Nominal Pipe	Design Bell Reinforcement	Maximum Reinforcement Under Tolerance							
Diameter	in² per foot	in² per foot							
15"	0.07	0.010							
18"	0.07	0.010							
24"	0.09	0.010							
30"	0.12	0.010							
36"	0.14	0.010							
42"	0.16	0.010							
48"	0.19	0.011							
54"	0.21	0.012							
60"	0.23	0.0135							
66"	0.26	0.015							
72"	0.28	0.0165							
78"	0.30	0.018							
84"	0.33	0.0195							
90"	0.35	0.021							
96"	0.37	0.0225							
102"	0.40	0.024							
108"	0.42	0.0255							

Allowable Tolerance For Last Full Wrap Of Reinforcing When Using Single Elliptical Cage The Last Full Wrap Of Reinforcing Shall Extend To The Shoulder Alternate Arrangement Of Point And Meet ASTM C-76 Bell Reinforcement 1⁄2" Min. Cover ∰ 3°Max Min. Cover - Rubber Gasket (Round Or Profile) (Round Rubber Gasket Shown)

> * All circumferential steel located above this line within 1.75 L is defined as bell reinforcement.

ROUND RUBBER GASKET SHOWN DETAIL OF BELL & SPIGOT CONCRETE PIPE JOINT USING ROUND OR PROFILE RUBBER GASKET

Class NS Concrete 12" For Pipes 14"x23" Through 19"x30" Any Wire Mesh Arrangement Which 24" For Pipes 24"x38" And Larger Provides 0.126 Square Inches Of Steel Area Per Linear Foot Both Ways May Be Used; Provided The Wires Are Spaced A Minimum Of 2" And/Or A Maximum Of 6" On Centers



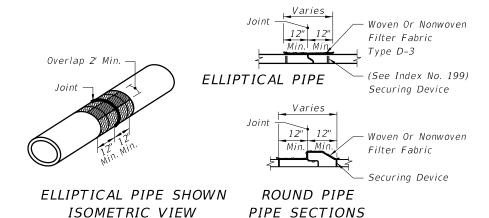
Filter Fabric Jacket Required PREFORMED PLASTIC JOINT (BEFORE PULL-UP)

Primer

Filter Fabric Jacket Required PROFILE RUBBER GASKET (BEFORE PULL-UP)

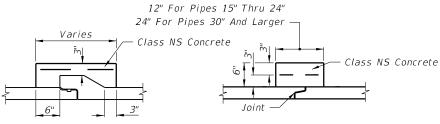
Cost of concrete jacket or filter fabric jacket to be included in cost of elliptical concrete pipe culverts.

ELLIPTICAL CONCRETE PIPE JOINTS



Cost of filter fabric jacket to be included in cost of pipe culverts.

FOR ALL PIPE TYPES - CONCRETE PIPE SHOWN FILTER FABRIC JACKET



Note: For reinforcement see elliptical pipe concrete jacket. (All Pipe Sizes)

TONGUE & GROOVE BELL AND SPIGOT

DISSIMILAR JOINTS

Bituminous Coating Required For CMP (Any Suitable Bituminous Material May

Be Field Applied) Bituminous Coating To

Extend 12" Beyond Concrete Collar

(Corrugated Steel, Corrugated Aluminum,

Corrugated Polyethylene, or PVC)

Note: Cost of concrete and bituminous coating to be included in contract unit price for either new pipe or Mitered End Section.

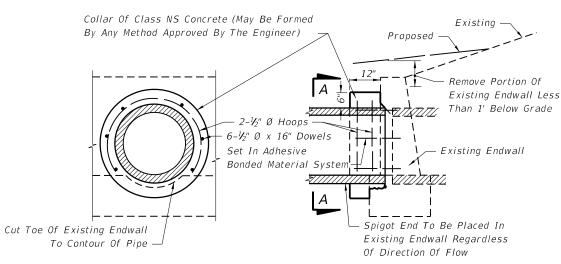
Alternate connection must be approved by the State Drainage Engineer.

A concrete jacket shall not be used to join:

a) metal pipe of dissimilar materials b) flexible pipe when the minimum cover required in accordance with Index No. 205 cannot be obtained

DISSIMILAR TYPES

CONCRETE JACKET FOR CONNECTING DISSIMILAR TYPES OF PIPE AND CONCRETE PIPES WITH DISSIMILAR JOINTS

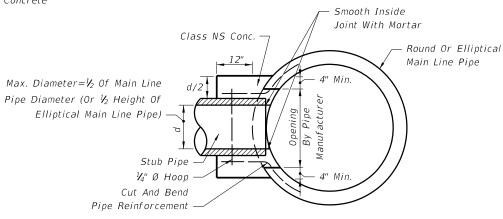


SECTION AA

LONGITUDINAL SECTION

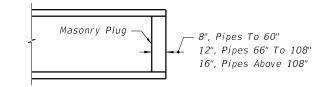
Note: Cost for removal and disposal of portions of top and toe of existing endwall and cost of concrete, reinforcing steel and construction of collar to be included in the contract unit price for pipe culvert.

CONCRETE COLLAR FOR EXTENSION OF EXISTING PIPE CULVERTS



Cost of concrete and steel to be included in contract unit price for pipe culvert.

CONCRETE COLLAR FOR JOINING MAINLINE PIPE AND STUB PIPE



Note: Unless otherwise called for in the plans, the cost of plugging pipes to be included in contract unit price for new pipe.

PIPE PLUG

FDOT DESIGN STANDARDS 2013

Class NS Concrete

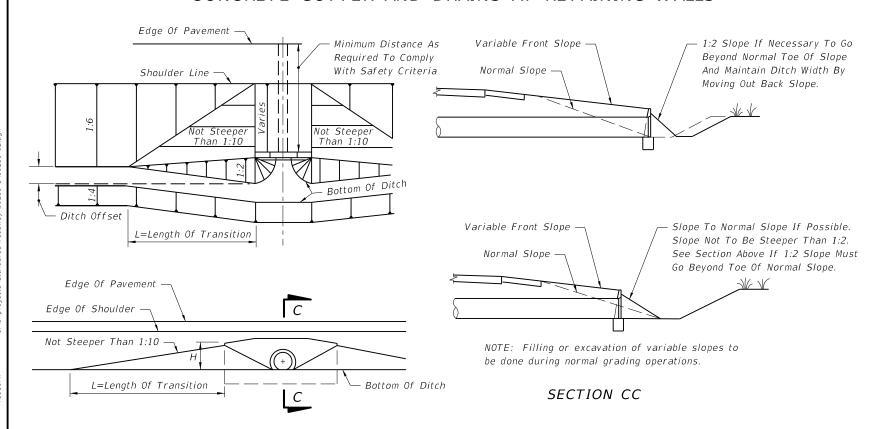
MISCELLANEOUS DRAINAGE DETAILS

SHEET *INDEX* NO. NO. 280

LAST REVISION 01/01/11

Note: PVC pipe, Schedule 40, to be paid for under the contract unit price for Polyvinyl Chloride Pipe Culvert (4"), LF.

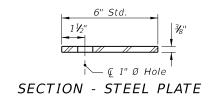
CONCRETE GUTTER AND DRAINS AT RETAINING WALLS

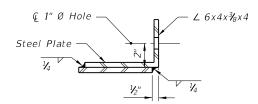


Use Larger Value Of Either:

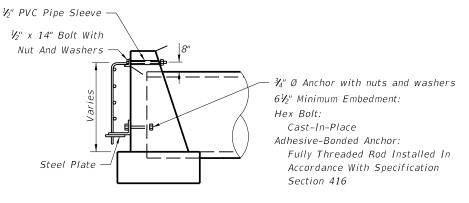
- 1. L=10xH (No Maximum)
- 2. L=10xDitch Offset (Maximum L=100')

METHOD FOR SETTING LIMITS OF VARIABLE FRONT SLOPES AT DRAINAGE STRUCTURES

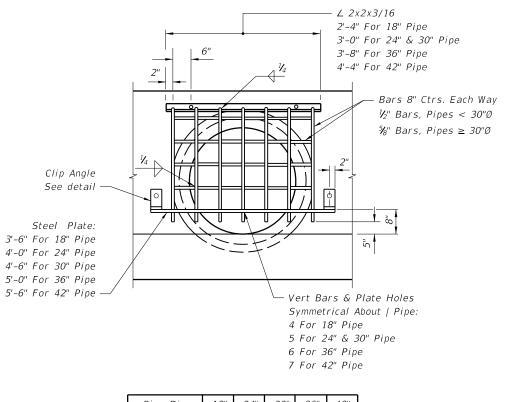




CLIP DETAIL



SIDE VIEW



Pipe Dia.	18"	24"	30"	36"	42"	
Grate (Lbs.)	48	58	74	90	111	

FRONT VIEW

Note: Guards to be constructed only at locations specifically called for in plans. Guard, plate & clips, bolts, nuts and sleeves to be included in the contract unit price for Reinforcing Steel (Miscellaneous).

GUARD AT PIPE ENDS

LAST REVISION 07/01/07

DESCRIPTION:



FDOT DESIGN STANDARDS 2013

MISCELLANEOUS DRAINAGE DETAILS

INDEX SHEET NO.

280

14
9

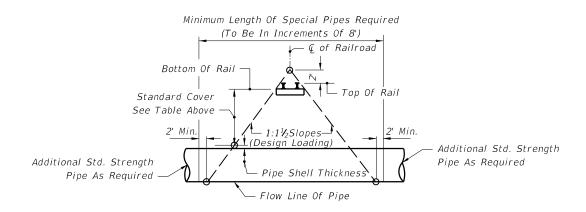
LAST

REVISION

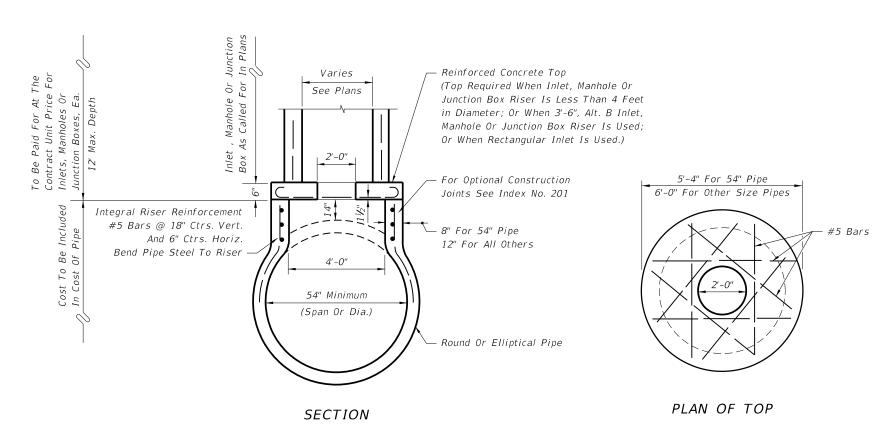
07/01/07

CLEARANCE	STRENGTH			
BELOW BOTTOM OF RAIL (FEET) ⁽²⁾	ASTM (C76) CLASS			
5.5	IV			
5.5 / 4.5 (1)	V			
5.5	V			
5.5 / 4.5 (1)	V			
5.5	V			
5.5	IV			
5.5	V			
5.5	V			
5.5 / 4.5 (1)	V			
5.5	IV			
6.0	V			
6.0	V			
5.5 / 4.5 (1)	V			
5.5	V			
	BELOW BOTTOM OF RAIL (FEET) ⁽²⁾ 5.5 5.5 / 4.5 ⁽¹⁾ 5.5 5.5 5.5 5.5 5.5 6.0 6.0 6.0			

- (1) Distance standard for yard and industrial tracks.
- (2) Clearance is for casing pipe. All subgrade carrier pipelines and wirelines will be installed within a casing pipe which will extend from Right-of-Way line to Right-of-Way line.

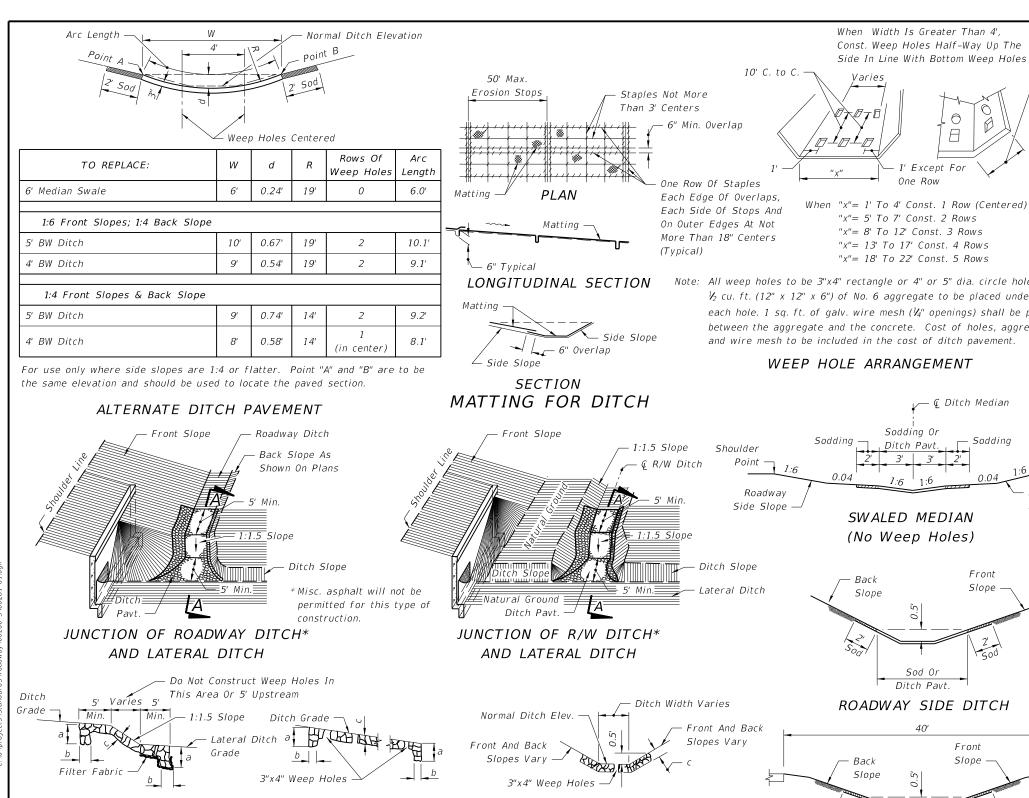


METHOD FOR DETERMINING THE LENGTH OF SPECIAL PIPE REQUIRED UNDER RAILROADS



INLETS, MANHOLES OR JUNCTION BOXES ON INTEGRAL PRECAST CONCRETE RISER FOR CONCRETE PIPE

MISCELLANEOUS DRAINAGE DETAILS



Note: All weep holes to be 3"x4" rectangle or 4" or 5" dia. circle hole. $\frac{1}{2}$ cu. ft. (12" x 12" x 6") of No. 6 aggregate to be placed under each hole. 1 sq. ft. of galv. wire mesh (¼" openings) shall be placed between the aggregate and the concrete. Cost of holes, aggregate and wire mesh to be included in the cost of ditch pavement.

 \bigcirc

— ℚ Ditch Median

Front

Slope

Front

Slope

1' Except For

One Row

WEEP HOLE ARRANGEMENT

Back

Slope

Back

Slope

Soddina Or

SWALED MEDIAN (No Weep Holes)

Sod Or

Ditch Pavt.

40'

4' Std.

Ditch Pavt.

40' MEDIAN

PLAN

SECTION EE

Standard Paved Ditch

Pavement

Sod

Flow Line

Sodded Ditch Paved Ditch

1.0' Deep

Filter Fabric

1.0' Deep

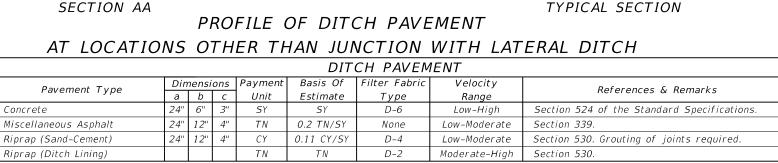
GENERAL NOTES

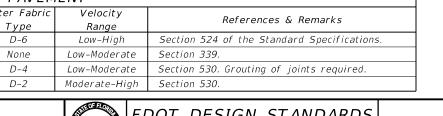
PAVED DITCH END TREATMENT

- 1. Type of ditch pavement shall be as shown on plans.
- 2. In concrete ditch pavement, contraction joints are to be spaced at 25' maximum intervals, or as directed by the Engineer. Contraction joints may be either formed (construction joint) or tooled. No open joints will be permitted in concrete ditch pavement.

Expansion joints with 1/2" preformed joint filler shall be constructed at all inlets, endwalls, and at intervals of not more than 200'.

- 3. Lip at end of ditch pavement shall normally be located downstream of DPI or on flatter grades where there is a decrease in ditch velocity.
- 4. Toewalls are to be used with all ditch paving. A toewall is not required adjacent to drainage structures.
- 5. When directed by the Engineer, weep hole spacing may be reduced to 5' minimum.
- 6. For junction of R/W ditch spillway and lateral ditch, sides of paving to be 1' high minimum.
- 7. For ditch pavements requiring filter fabric, the fabric shall be placed directly beneath the pavement for the entire length and width of the pavement. When weep holes with aggregate are used, the filter fabric shall be placed below the aggregate to form a mat continuous with or underlapping the pavement fabric. (See Index No. 199 for fabric type and application).
- 8. Ditch pavement requiring reinforcement shall be detailed in the plan.
- 9. Cost of plastic filter fabric to be included in the contract unit price for ditch pavement.
- 10. Sodding to be paid for under contract unit price for Performance Turf, SY







DOT DESIGN STANDARDS 2013

Shoulder

Point

Side Slope

SHEET *INDEX* NO. NO. 281

Varies (25' Min.)

Sodded Ditch

1.5' Deep

07/01/07

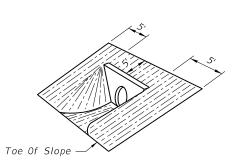
IAST REVISION

LAST

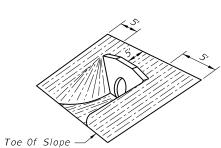
REVISION

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DESCRIPTION:



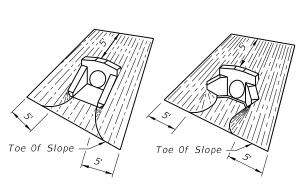
Note: Sodding quantities for each endwall to be determined by the designer from this detail.



(EXCEPT INDEX NO. 250) STRAIGHT ENDWALL

STRAIGHT ENDWALL INDEX NO. 250

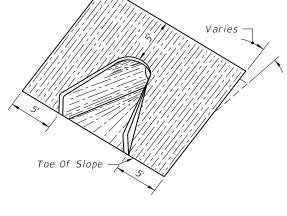
U-TYPE ENDWALL INDEX NO. 261



WINGED ENDWALLS

INDEX NO. 266

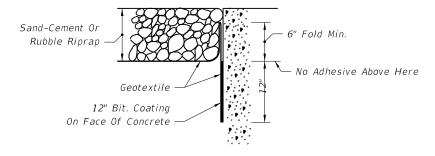
U-TYPE WINGS



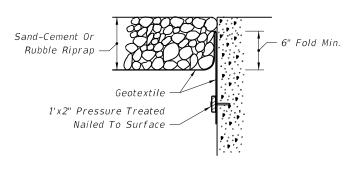
45° WINGS

FLARED END SECTION INDEX NO. 270

	SOD QUANTITIES (SY)																				
					IN	INDEX NO. 250							IND	INDEX NO. 261 INDEX NO. 266						INDEX NO. 270	
PIPE						SLC)PE						SLOPE				SLOPE				ALL SLOPES
SIZE		1:2			1:3			1:4			1:6		1:2	1:3	1:4	1:6	1:2	1:3	1:4	1:6	ALL SLOTES
						PIF	PES							PIPE	5			PIF	PES		PIPES
	1	2	3	1	2	3	1	2	3	1	2	3	1	1	1	1	1	1	1	1	1
12"																	14	15	18	22	10
15"	19	21	24	22	26	29	26	30	33	34	38	43	13 (15)	16	17	23	15	17	20	25	11
18"	21	24	27	25	29	33	30	34	38	39	44	50	14 (16)	17	19	25	16	18	22	28	11
21"																					12
24"	26	30	34	32	37	42	38	44	50	50	58	66	15 (17)	19	21	28	19	22	26	34	14
27"																					15
30"	31	37	42	39	46	53	46	55	63	62	74	85	17 (18)	21	24	32	21	25	30	40	16
36"	37	44	52	46	56	65	56	67	79	76	91	107					24	29	35	47	18
42"	43	53	62	55	67	79	67	82	96	91	111	132					27	32	39	54	19
48"	50	62	73	64	79	93	78	97	115	108	133	158					30	36	44	61	21
54"	57	71	85	74	92	110	91	113	136	126	157	188									21
60"																					22
66"																					25
72"																					26
												·	() Endw	all Wi	th Baf	fles					



BONDED OPTION



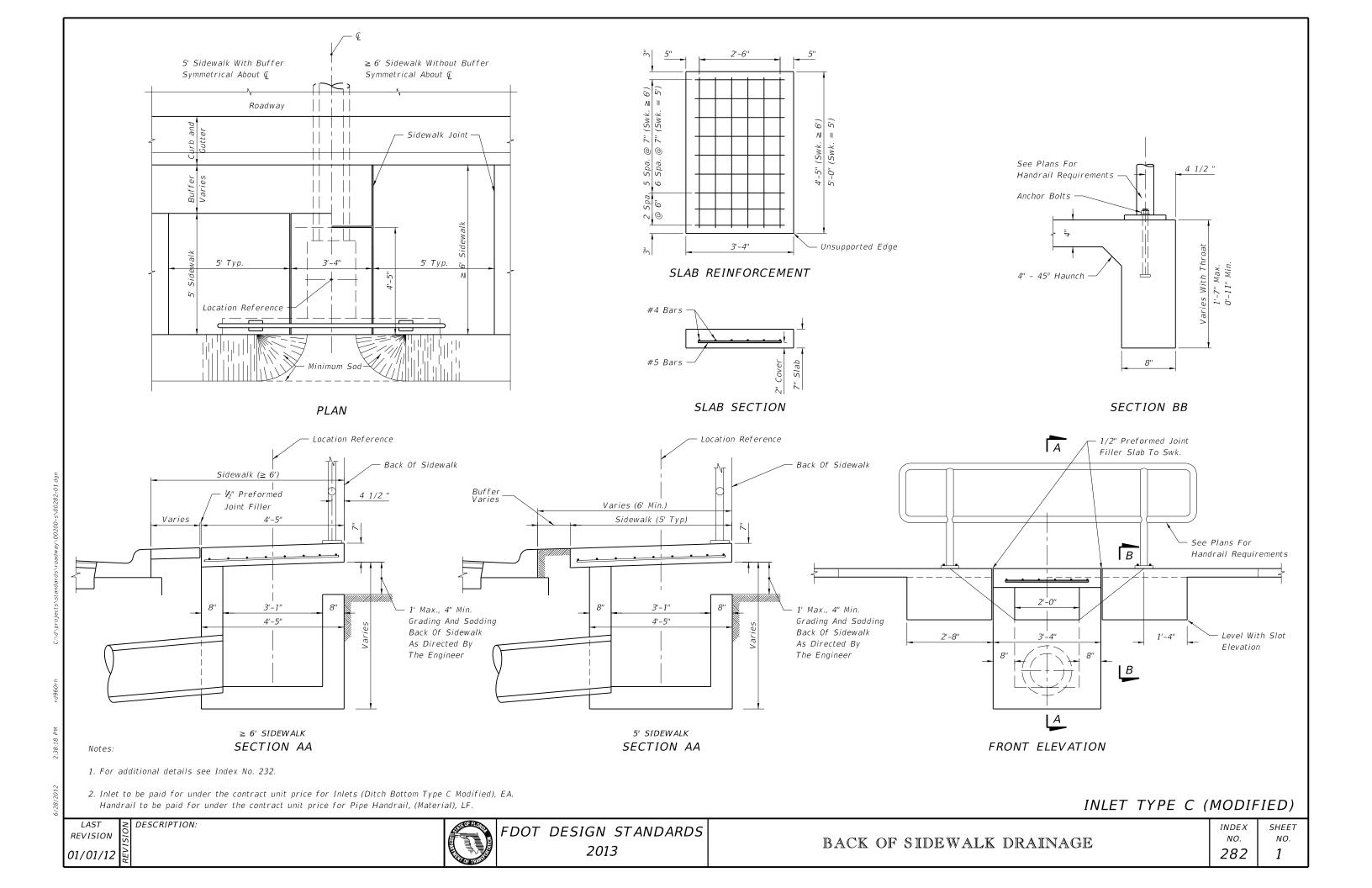
NAILED OPTION

Note: Either option may be used unless otherwise called for in the plans.

GEOTEXTILE PLACEMENT AT CONCRETE STRUCTURE

SOD

FDOT DESIGN STANDARDS 2013



Pipe Size (in)	С	Concrete Class I (CY)	Sand-Cement Riprap (CY)
15	4'-9"	2.3	1.1
18	5'-3"	2.6	1.3
24	6'-3"	3.3	1.8

SPECIAL CONCRETE ENDWALL

under the contract unit price for Pipe Handrail, (Material), LF. LAST REVISION 07/01/09

Notes:

1. Maximum pipe size shall be 24" diameter.

DESCRIPTION:

2. Grading back of sidewalk varies and shall be done as directed by the Engineer.

3. Concrete quantities shown are for maximum wall heights, and shall be basis for estimate and payment. 4. Riprap quantities shown are for estimate purposes only. Cost of riprap to be included in cost of the endwall. 5. Endwalls to be paid for under the contract unit price for Concrete Class I (Endwalls), CY. Handrail to be paid for



FDOT DESIGN STANDARDS 2013

Sidewalk

1/2" Preformed Joint Filler

Riprap-Half Plan Shown

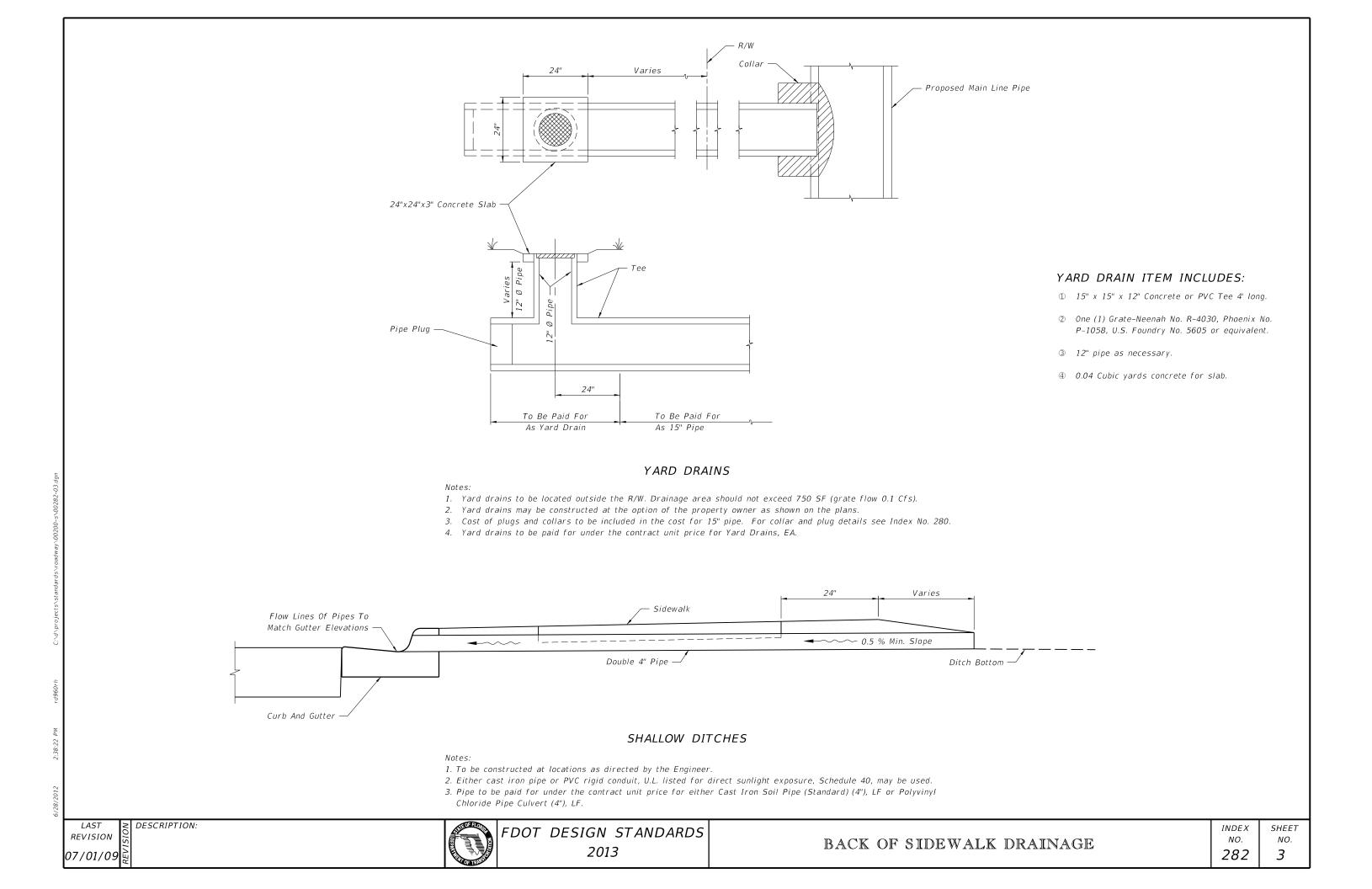
Sidewalk

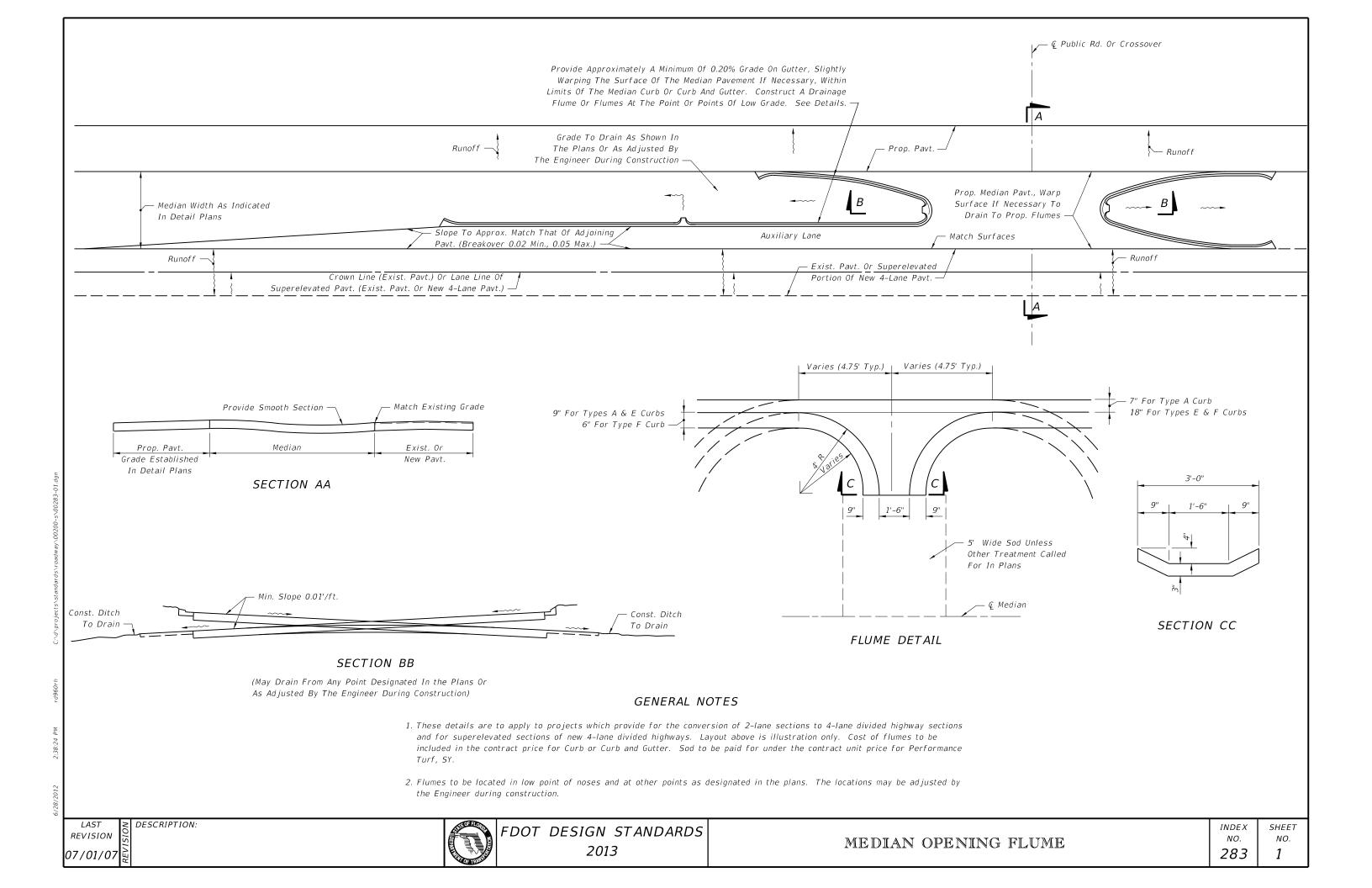
See Plans For Handrail Requirements

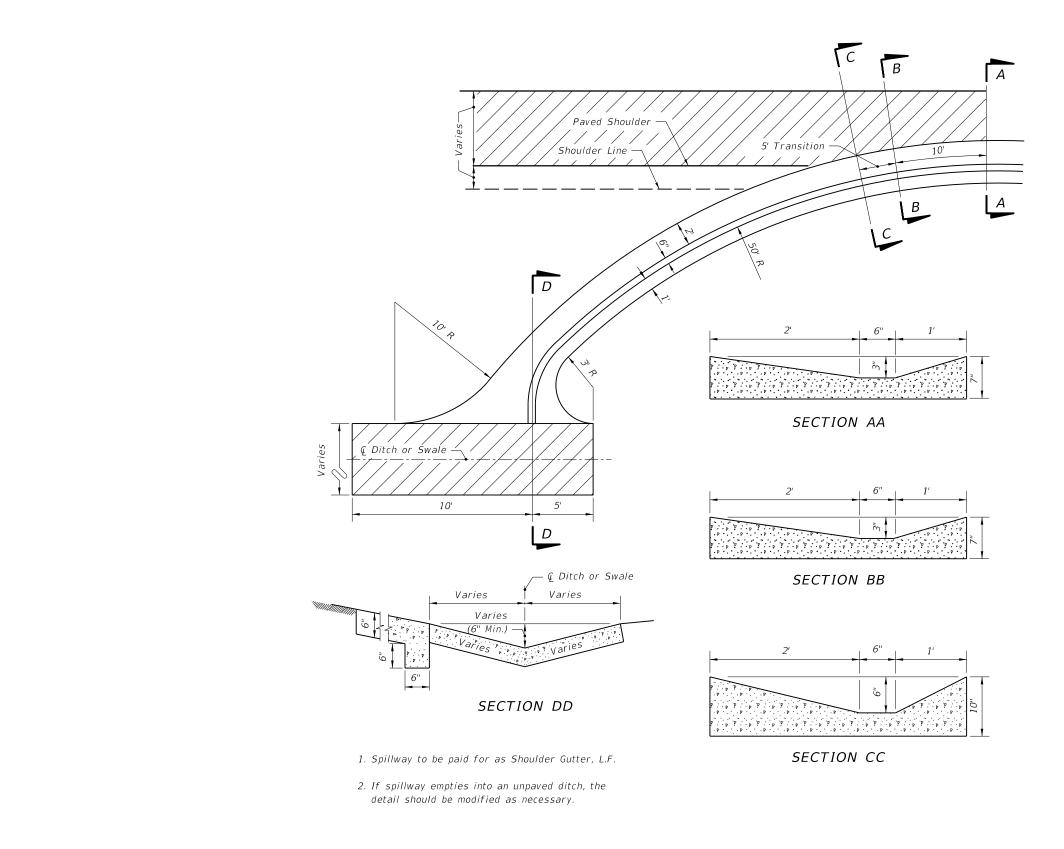
– Riprap Slopes Steeper Than 1:1½ (Max. 1:1),

And Ditch Bottom (Symmetrical About Q)

SHEET NO. NO. 282



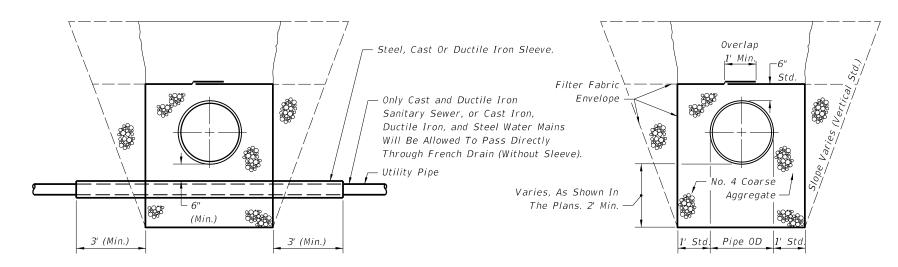




DETAIL OF CONCRETE SPILLWAY AT END OF SHOULDER GUTTER (TO BE USED WHERE INLETS, PIPES & ENDWALLS ARE IMPRACTICAL)

LAST REVISION 07/01/09





ROUND PIPE SHOWN
UTILITY PIPES THRU FRENCH DRAIN

ROUND PIPE SHOWN
STANDARD CROSS SECTION (ENLARGED)

FRENCH DRAIN SYSTEM

GENERAL NOTES

- 1. Pipe shall be any of the optional types permitted in Section 443 of the Specifications unless otherwise restricted in the plans. Dissimilar types of pipe will not be permitted in a continuous run of pipe.
- 2. Concrete pipe shall be placed with the slots positioned on sides.
- 3. Alignment joints are standard (gaskets not required). Recorrugation of metal pipe ends not required.
- 4. The contractor may submit other methods of providing slots having equal or greater area of opening, for approval by the Engineer.
- 5. Filter fabric shall be Type D-3 meeting the requirements of Section 985. All filter fabric joints shall lap a minimum of one (1) foot.
- 6. The standard cross section shall be constructed unless other section(s) described or detailed in the plans.
- 7. For supplemental details see Index No. 280.
- 8. The contractor shall take the necessary precautions to prevent contamination of the trench with sand, silt and foreign materials.
- 9. French drains shall be paid for under the contract unit price for French Drains, LF. The unit price shall include the cost of pipe, pipe plugs, pipe fittings, coarse aggregate and filter fabric in place, and the cost for trench excavation, backfill and compaction. The unit price shall also include the cost for disposal of surplus excavated materials and cost for restoration of pavement removed or damaged by french drain construction, but shall not include payments for items paid for elsewhere.

DESIGN NOTES

- 1. Pipe invert should be at or above the water table whenever possible.
- 2. French drains with minor dimensional changes or otherwise different from the standard cross-section shall be either described or detailed in the plans. French drains with significantly different cross-sections shall be detailed in the plans.

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6/28/2012

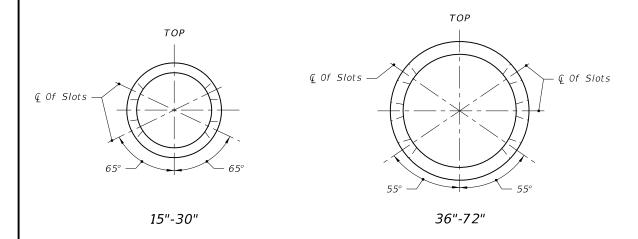
LAST DESCRIPTION:
REVISION 55
07/01/10



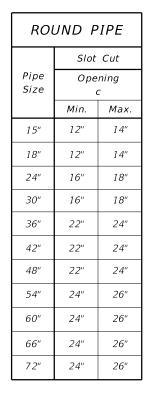
FDOT DESIGN STANDARDS 2013 INDEX NO.

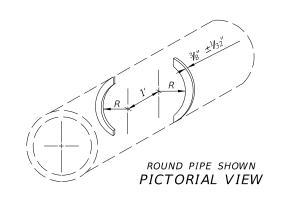
FRENCH DRAIN

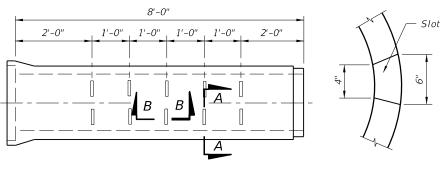
INDEX SHEET NO. NO. 285 1



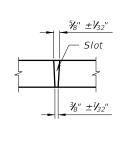
ELLIPT	ICAL	PIPE
	Slot	Cut
Pipe Size	•	ning :
	Min.	Max.
14"x23"	10"	12"
19"x30"	14"	16"
24"x38"	14"	16"
29"x45"	20"	22"
34"x53"	20"	22"
38"x60"	20"	22"



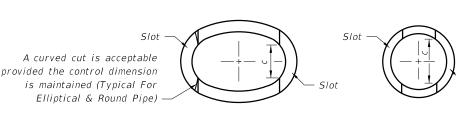




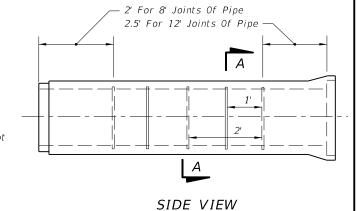
SIDE VIEW SECTION AA OPTION A - ROUND PIPE



SECTION BB



ELLIPTICAL PIPE ROUND PIPE SECTION AA



OPTION B - ROUND OR ELLIPTICAL PIPE

SLOTTED PIPE OPTIONS

REVISION 07/01/07



Fine Aggregate

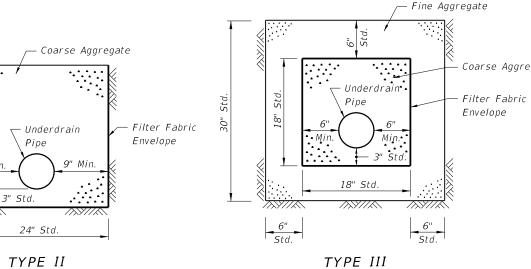
Underdrair

18" Std.

TYPE I

DESCRIPTION:





DESIGN NOTES

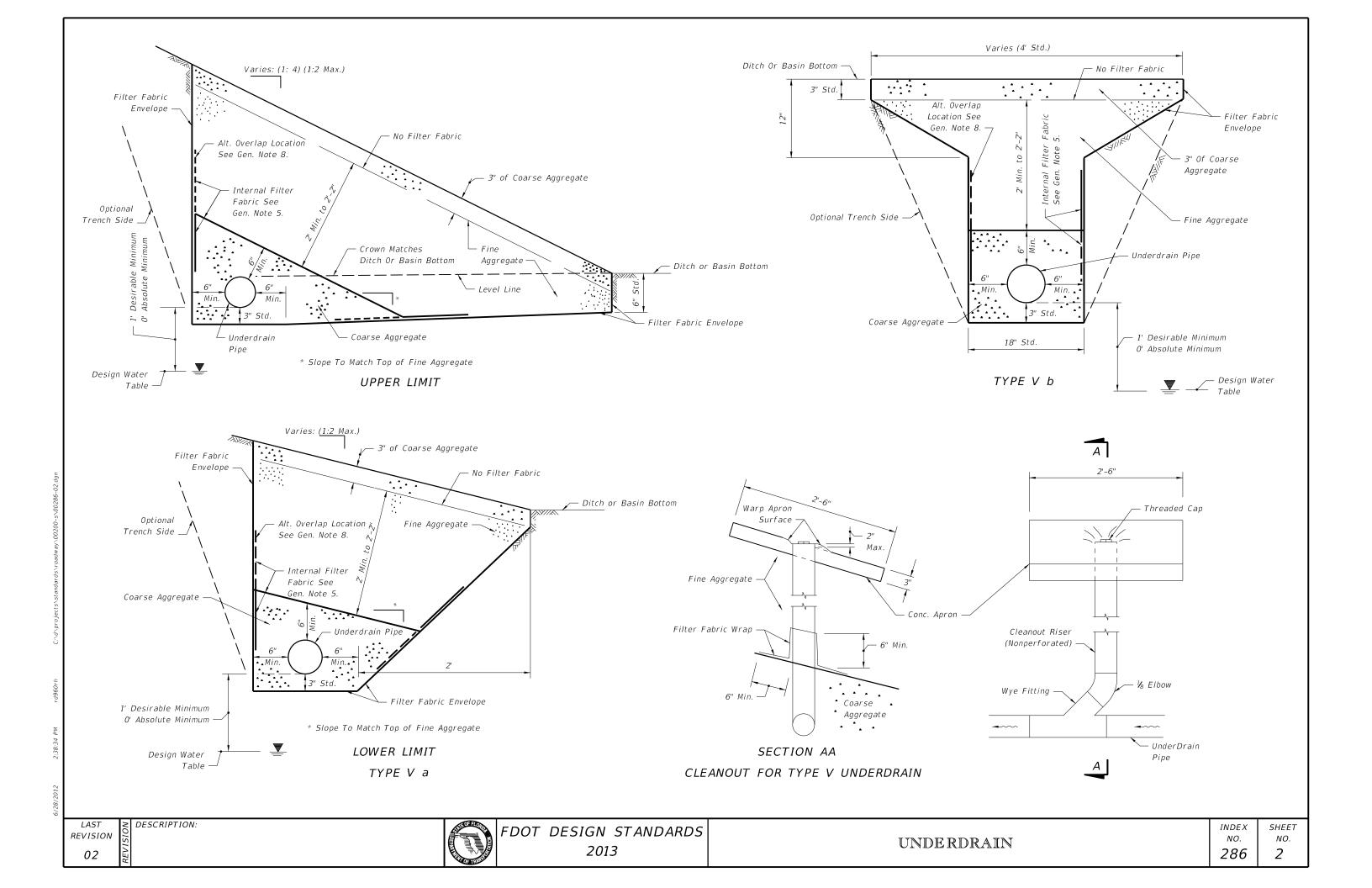
- 1. The type of underdrain should be selected to meet design water removal rate and soil conditions. Caution is prescribed in the use of these typical sections since special designs may be required to satisfy project conditions.
- 2. Type I underdrain is intended for minimum water removal conditions.
- 3. Type II underdrain is intended for moderate water removal conditions. Where reactive conditions may create chemical clogging, the use of an inert material and/or elimination of the filter fabric may be necessary.
- 4. Type III underdrain is intended for maximum water removal conditions. Filter fabric is required between the coarse aggregate or fine aggregate including those described in general notes 2 and 3. Design note 3 applies for reactive conditions.
- 5. Type V underdrain is intended for use in detention basins and other locations which require a filtration system. The standard fine aggregate specified for Type V underdrain conforms to filtration gradation requirements of Chapter 62-25 FAC.
- 6. The designer should detail in the plans, the location of: (a) Type V underdrain, (b) nonstandard locations of Type I, II, and III underdrain, (c) underdrain inspection boxes, (d) cleanouts for Type V underdrain, and (e) underdrain outlet pipes.
- 7. The designer should specify the flow line elevations at the beginning, bends, junctions and ends of underdrain pipes and outlet pipes.
- 8. The designer should evaluate whether an external filter fabric envelope is required around underdrain Types I and III. When required, fabric shall be specified in the plans.

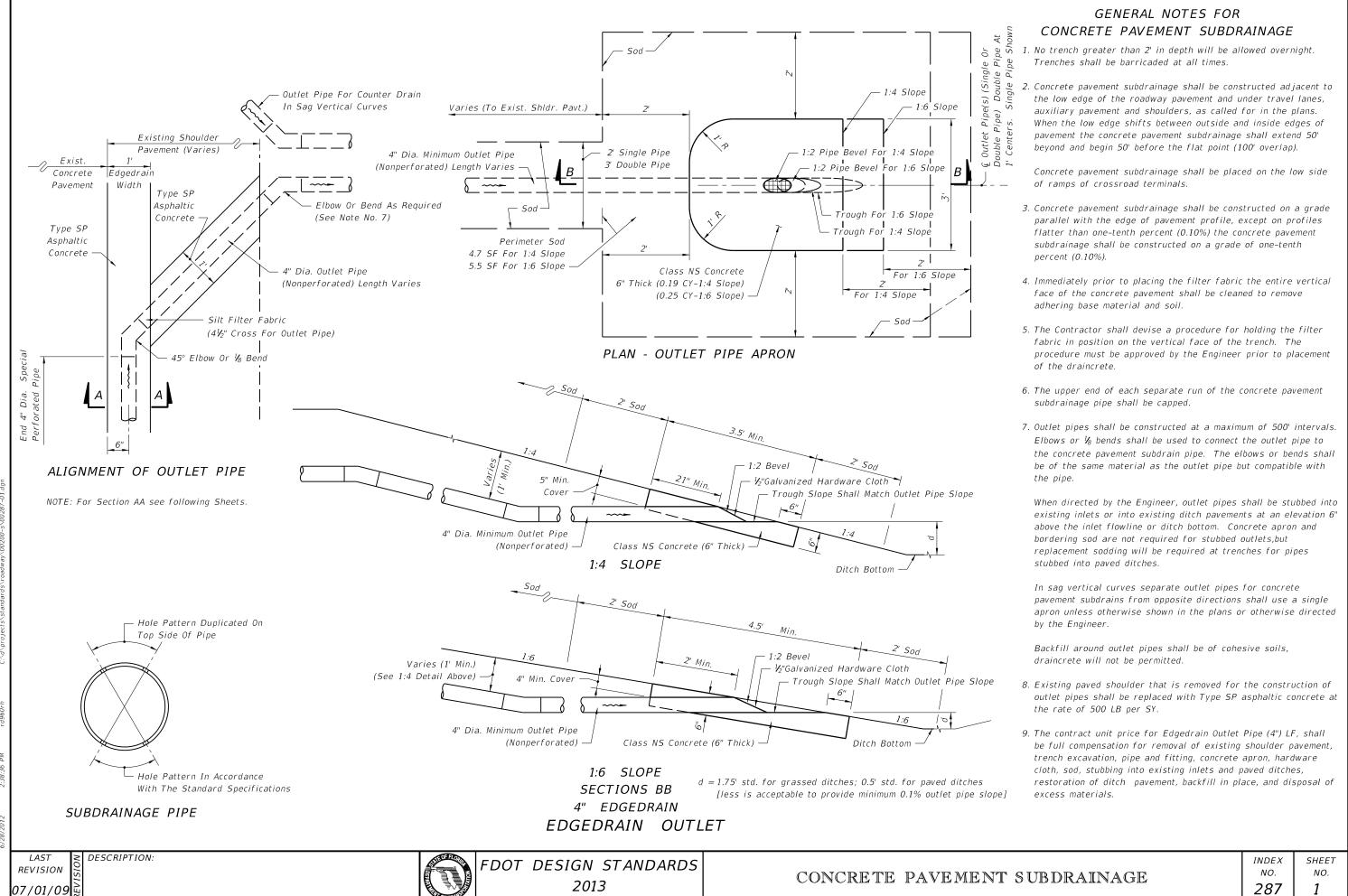
GENERAL NOTES

- 1. The underdrain pipe shall be either 4" smooth or 5" corrugated tubing unless otherwise shown in the plans. The size to be furnished will be based on the nominal internal diameter of a pipe with a smooth interior wall. Except when prohibited by the plans, the special provisions or this standard, pipe with a corrugated interior wall may be provided based on the following size equivalency.
 - 4" smooth interior equivalent to 5" corrugated interior
 - 5" smooth interior equivalent to 6" corrugated interior
 - 6" smooth interior equivalent to 8" corrugated interior
 - 8" smooth interior equivalent to 10" corrugated interior
- 2. Fine aggregate shall be quartz sand meeting the requirements of Sections 902-4 of the Standard
- 3. Coarse aggregate shall be gravel or stone meeting the requirements of Sections 901-2 or 901-3. The gradation shall meet Section 901, Grades 4, 467, 5, 56 or 57 stone unless otherwise shown restricted in the plans.
- 4. Underdrain Type I, II, III and V shall be in accordance with Section 440.
- 5. Filter fabric shall be Type D-3 (See Index No. 199). The internal filter fabric of Type V underdrain shall have a permittivity of 0.7 /sec. and an AOS of #40 sieve.
- 6. When Type I is used, a filter fabric sock meeting Section 948 is required.
- 7. See Index No. 500 for the standard location of Type I, II, and III underdrain. The location of Type V underdrain and nonstandard locations of Type I, II, and III underdrain will be as detailed in the plans.
- 8. All filter fabric joints shall overlap a minimum of 1'. The internal filter fabric of Type V underdrain shall overlap into the coarse aggregate or the fine aggregate a minimum of 1'.
- 9. Underdrain outlet pipes shall be nonperforated and all bends shall be made using V_8 (45 deg.) elbows. 90 deg. bends shall be constructed with two $\frac{1}{8}$ elbows separated by at least 1' of straight pipe. Outlet pipes stubbed into inlets or other drainage structures shall be not less than 6" above the structure flow line. Outlet pipes discharging to grassed areas shall have concrete aprons, hardware cloth, and bordering sod as shown in Index No. 287 for Edgedrain outlets.
- 10. Pay Item shall be based on the size of the smooth interior products. The contract unit price for Underdrain, LF, shall include the cost of pipe, fittings, aggregate, sock, filter fabric, underdrain cleanouts, and concrete aprons.

The contract unit price for Underdrain Outlet Pipe, LF, shall be full compensation for trench excavation, pipe and fittings, concrete aprons, hardware cloth for concrete aprons, stubbing into drainage structures, backfill in place, and disposal of excess materials.

The contract unit price for Underdrain Inspection Box, EA. shall be for the number completed and accepted.





1. The edgedrain sections for DRAINCRETE SUBDRAINAGE are applicable to pavement construction identified as RIGID PAVEMENT on Index No. 505, Sheet 2 and 4.

2. The contractor shall confine the construction of draincrete edgedrain to an area in which the entire operation can be carried out in five (5) work days, unless another construction period is called for in the plans, with sufficient time allowed for the draincrete to set before placement of pavement.

METHOD OF PAYMENT

NEW CONSTRUCTION:

1. The contract unit price for Edgedrain (Draincrete) LF shall be full compensation for trench excavation, disposal of excess material, filter fabric, draincrete edgedrain pipe and fittings and draincrete.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.

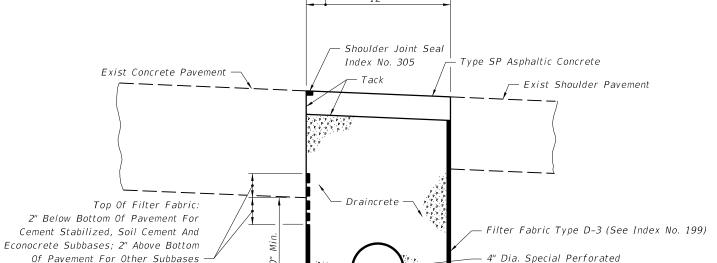
FOR REHABILITATION:

1. The contract unit price for Edgedrain (Draincrete) LF, shall be full compensation for removal of existing shoulder pavement, trench excavation, disposal of excess materials, filter fabric, draincrete edgedrain pipe and fittings, and draincrete, necessary for edgedrain construction.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.

Shoulder pavement shall be paid for under the contract unit price for Type SP, Asphaltic Concrete.

Shoulder joint seal shall be paid for under the contract unit price for Pavement Joint, LF.



REHABILITATION

12"

DRAINCRETE SUBDRAINAGE

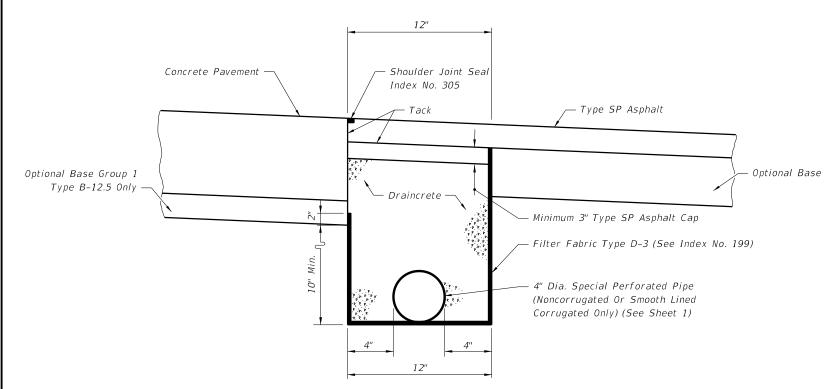
LAST REVISION 07/01/09

DESCRIPTION:



Pipe (Noncorrugated Or Smooth Lined Corrugated Only)

(See Sheet 1)



NEW CONSTRUCTION:

1. The contract unit price for Edgedrain (Draincrete) LF shall be full compensation for trench excavation, disposal of excess material, filter fabric, draincrete edgedrain pipe and fittings and draincrete.

METHOD OF PAYMENT

NOTES FOR DRAINCRETE

PAVEMENT SUBDRAINAGE

2. The contractor shall confine the construction of draincrete edgedrain to an area in which the entire

the plans, with sufficient time allowed for the draincrete to set before placement of pavement.

operation can be carried out in five (5) work days, unless another construction period is called for in

1. The edgedrain sections for DRAINCRETE SUBDRAINAGE are applicable to pavement construction

- Payment for outlet pipe shall be in accordance with General Note 9, Sheet 1 of 4.
- 2. Type B-12.5 shall be paid for under the contract unit price for Optional Base.

identified as RIGID PAVEMENT on Index No. 505, Sheet 2 and 3.

3. Shoulder pavement shall be paid for under the contract unit price for Type SP, Asphaltic Concrete.

ASPHALT BASE SUBDRAINAGE

ASPHALT SHOULDERS

LAST REVISION

DESCRIPTION:



FDOT DESIGN STANDARDS 2013 INDEX SHEET NO. NO. 287 3

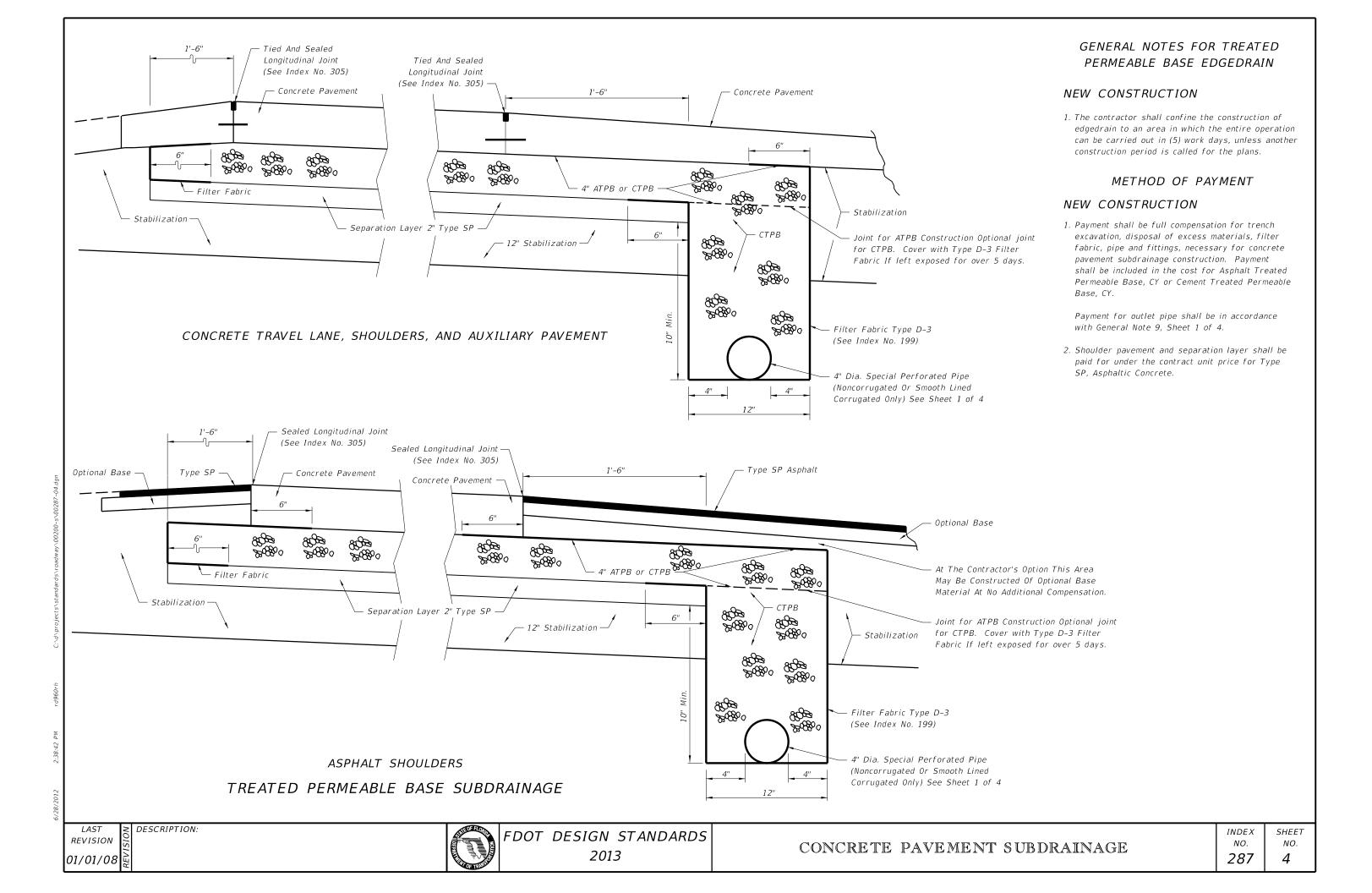
CONCRETE PAVEMENT SUBDRAINAGE

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rd960rh

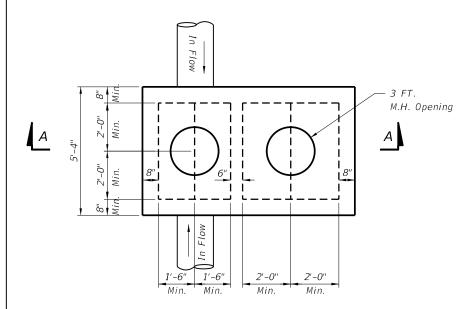
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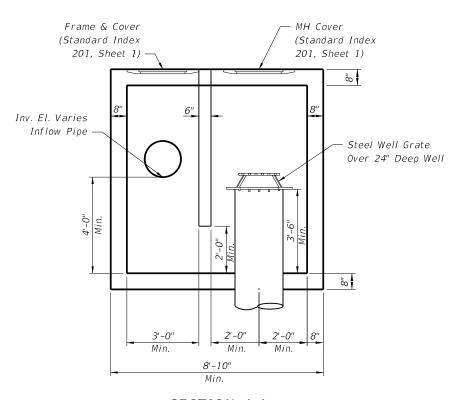


STEEL WELL GRATE DETAIL NEENAH GRATE CATALOG NO. R-4341-A OR EQUIVALENT

Note: Steel well grate to be installed over 24" deep well. Steel grate to be hot dipped galvanized after fabrication. Cost to be included in the price of casing.

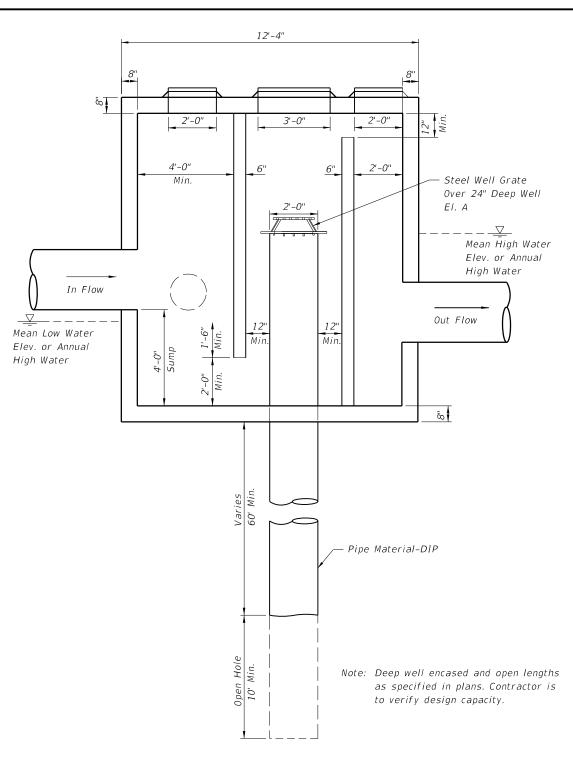


TOP SLAB PLAN



SECTION A-A

STRUCTURE WITH NO OUTFLOW



SPECIAL MANHOLE STRUCTURE DETAIL WITH OUTFALL

Design Notes

- 1. Depth of Casing Varies, 60' min.
- 2. Depth of Open Hole, 10'-20'.
- 3. Actual Size Of The Inflow And Outflow Chambers Will Be Determined By The Size Of The Pipes (Refer To Table 3 Of Index 200.) The Width Of The Box Shall Be Constant Based On The Largest Pipe. The Length Is To Be Adjusted Based On Size and Orientation Of The Pipes.

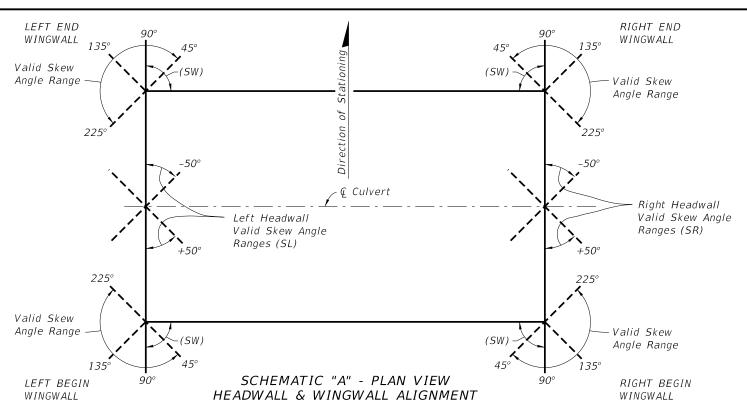
LAST REVISION 01/01/09



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REVISION

01/01/11



NOTE: All headwall and culvert skew angles are measured in degrees from a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic "B".

Construction Joint in Footing permitted Exterior Barrel Wall Construction Joint Limits of sloped (See Detail "F", top surface (Lw) Sheet 5) Front Tip Construction Joint in Wingwall required - Front Tip ∖Exterior Barrel Wall -Front Tip Height (He) (1'-6" Min.) Construction Joint in Footing permitted Half Elevation showing Half Elevation showing Parallel Wingwalls Tapered Wingwalls

END ELEVATION
OF CULVERT

GENERAL NOTES:

LIVE LOAD: HL-93.

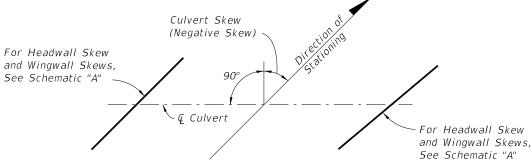
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 7.

REINFORCING STEEL: See the "Box Culvert Data Tables" in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.



SCHEMATIC "B" - PLAN VIEW CULVERT ALIGNMENT

NOTE: For Culvert Skew see Contract Plans.

TAB		NIMUM E IGITUDIN			
BAR	SPLICE (CLASS B)	BAR	SPLICE (CLASS B)
SIZE	CLASS II	CLASS IV	SIZE	CLASS II	CLASS IV
	(3400 psi)	(5500 psi)		(3400 psi)	(5500 psi)
#3	1'-0"	1'-0"	#8	3'-6"	2'-9"
#4	1'-4"	1'-4"	#9	4'-5"	3'-6"
#5	1'-8"	1'-8"	#10	6'-7"	4'-5"
#6	1'-11''	1'-11"	#11	7'-10"	6'-5"
#7	2'-8"	2'-3"			

TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.

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DESCRIPTION:

NOTE:
Construction Joints in wingwalls and footings are located as follows:
For non-skewed wingwalls they are located adjacent to the exterior
face of the exterior barrel wall; when the Q of wingwall and Q of
exterior barrel wall results in an acute angle see Left End Wingwall
above, and when the angle is obtuse see Left Begin Wingwall above
and Detail C (Sheet 5).

PART PLAN SHOWING PARALLEL WINGWALLS

AND LOCATION OF CONSTRUCTION JOINTS

THE STATE OF THE S

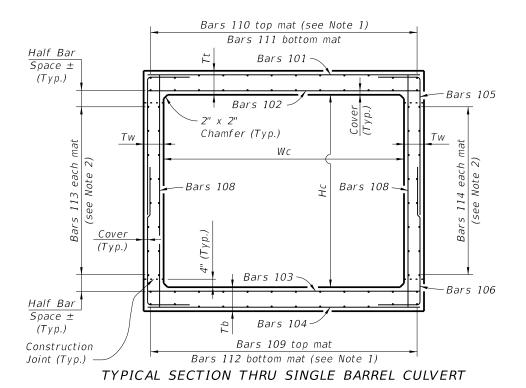
FDOT DESIGN STANDARDS
2013

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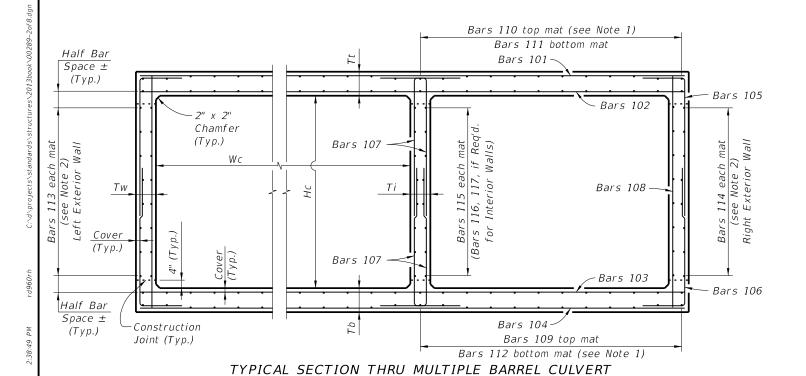
SHEET

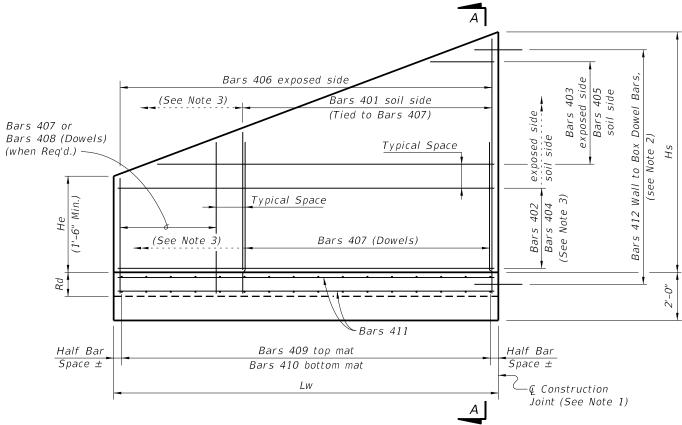
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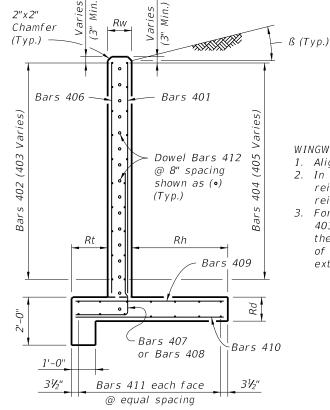
CULVERT BARREL NOTES:

- 1. Space Bars 110 and 112 with a bar in each corner, and at the $\c C$ of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4" clear.





WINGWALL ELEVATION - Variable Height (Left End shown - other corners similar)



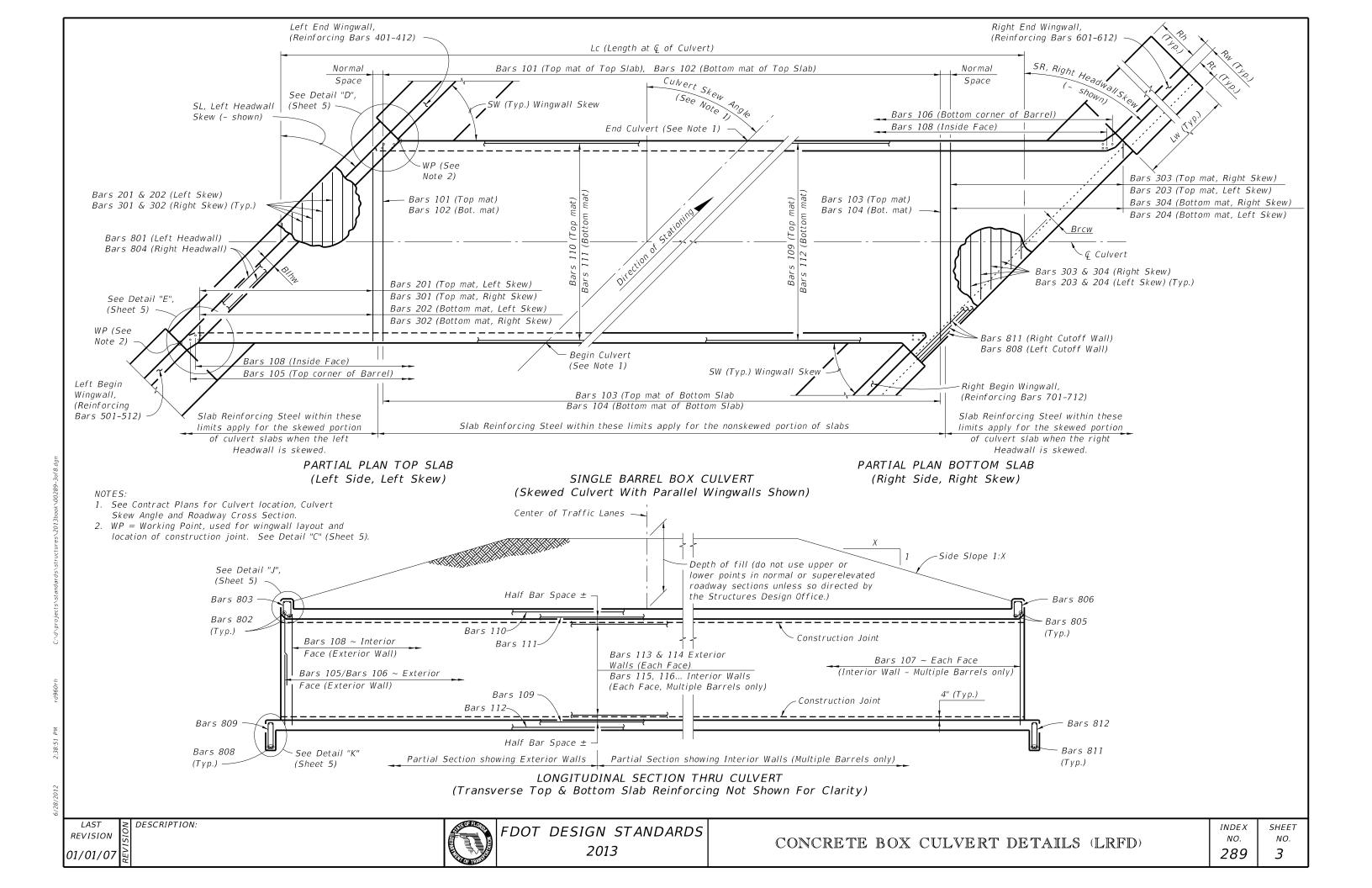
WINGWALL NOTES:

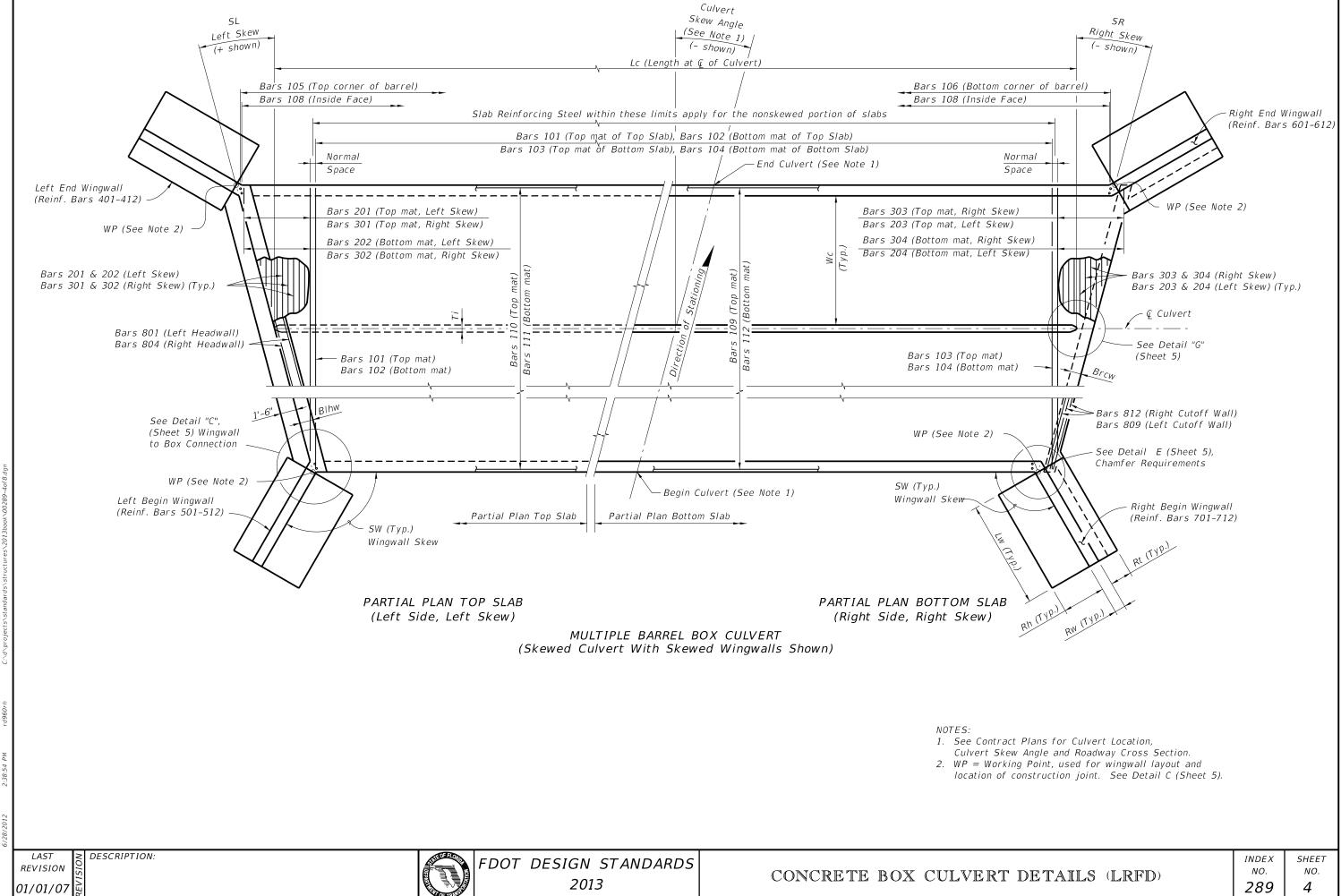
- 1. Align construction joint perpendicular to wingwall.
- 2. In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover.
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

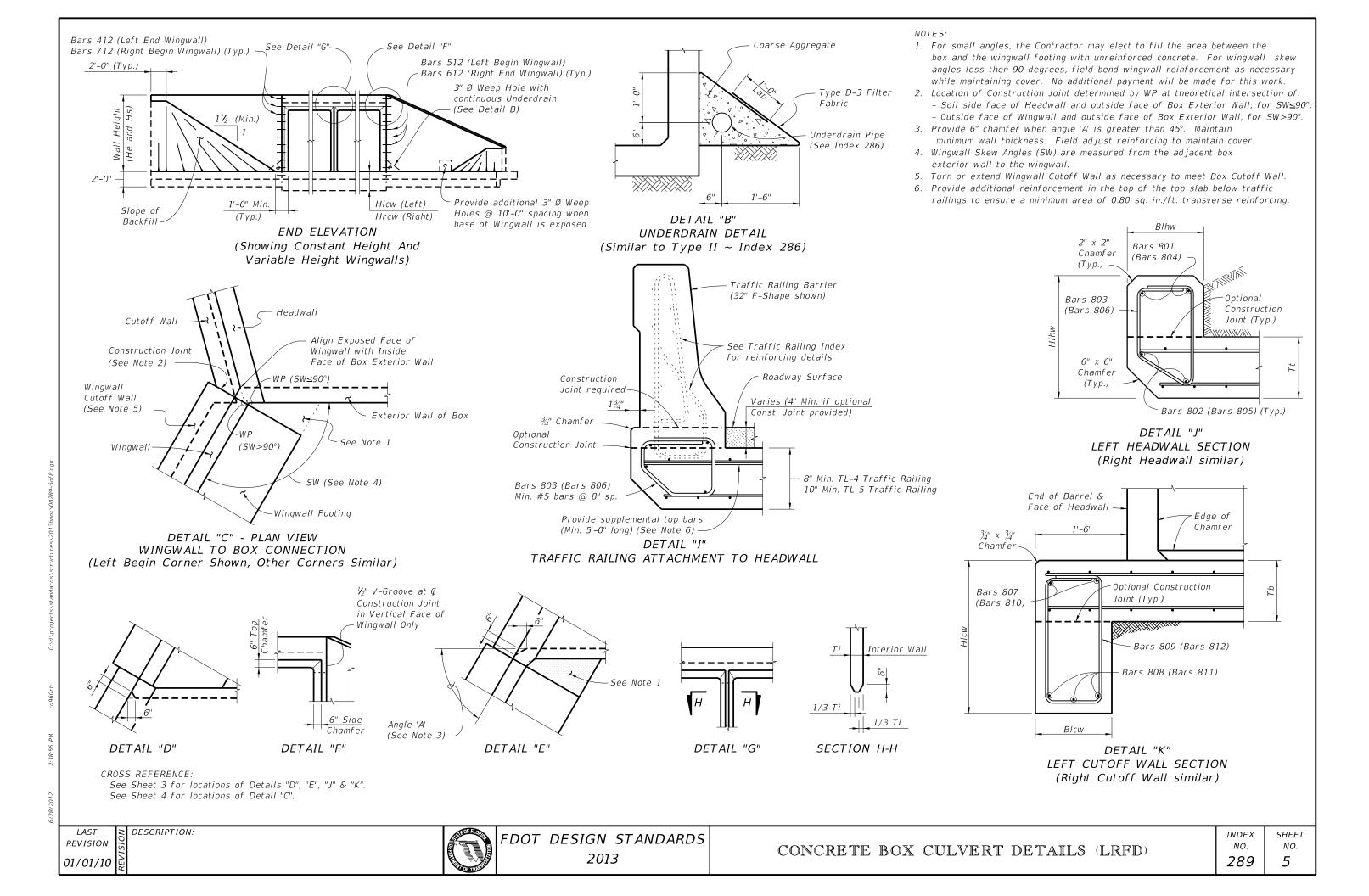
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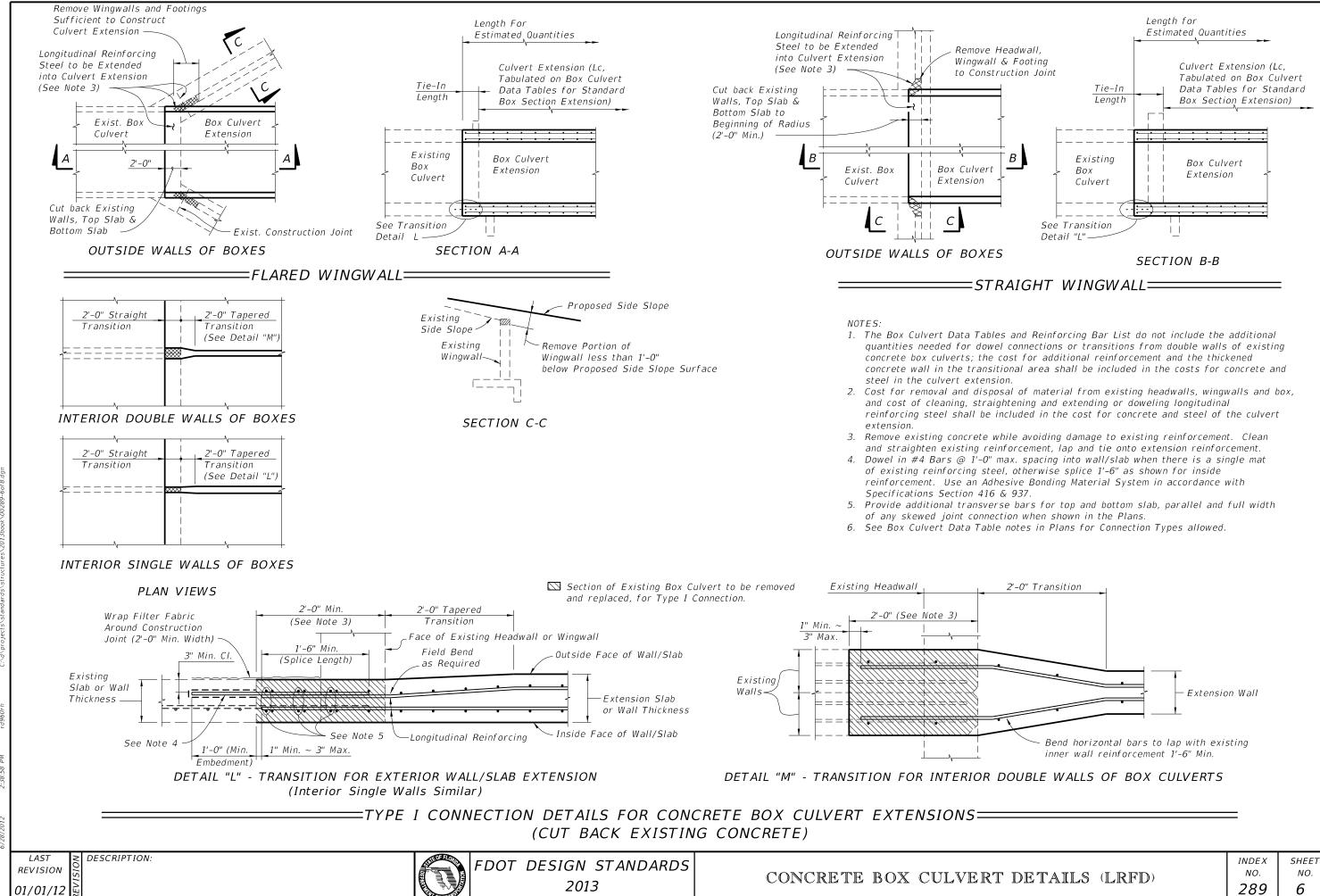
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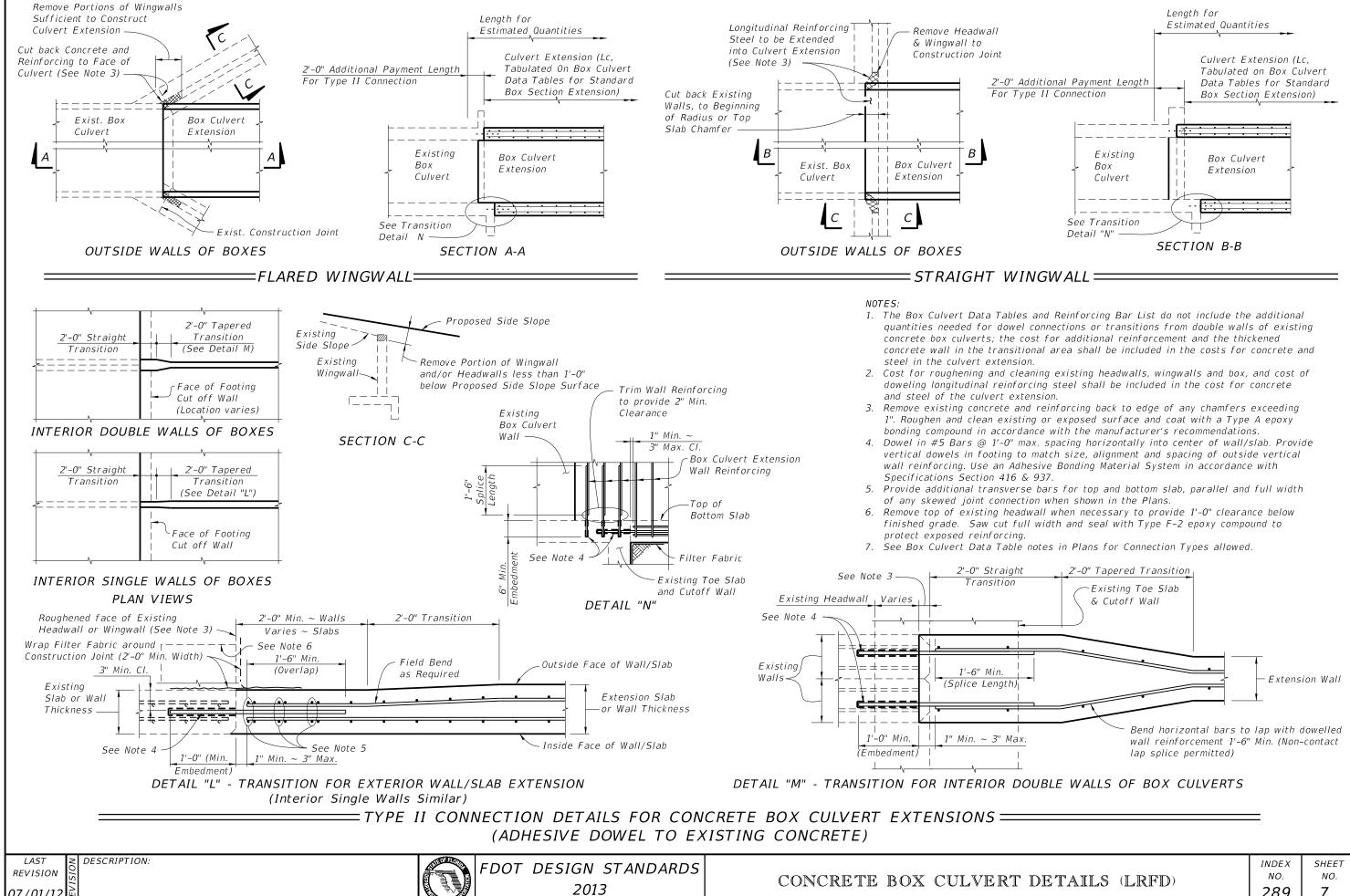
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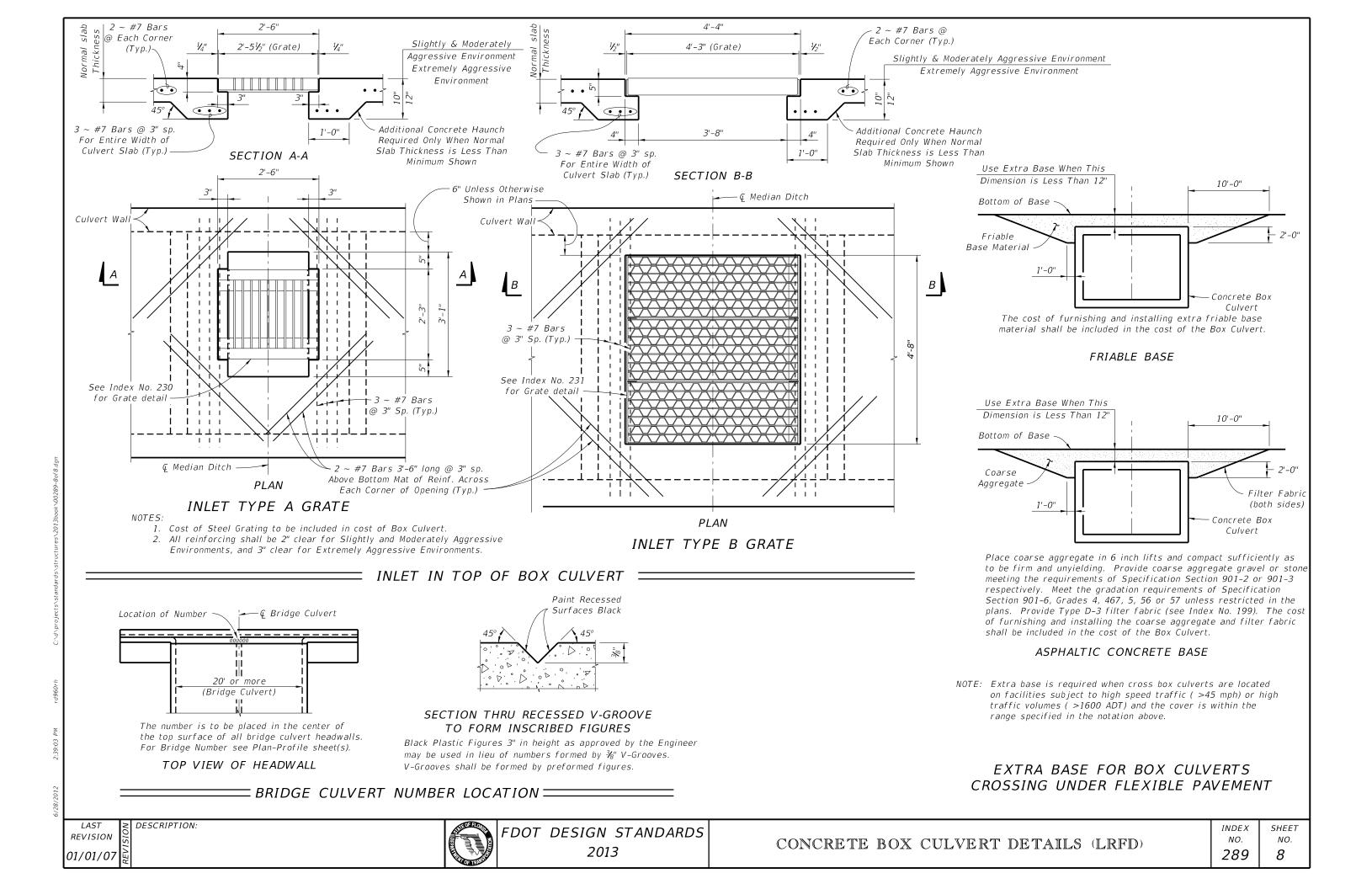


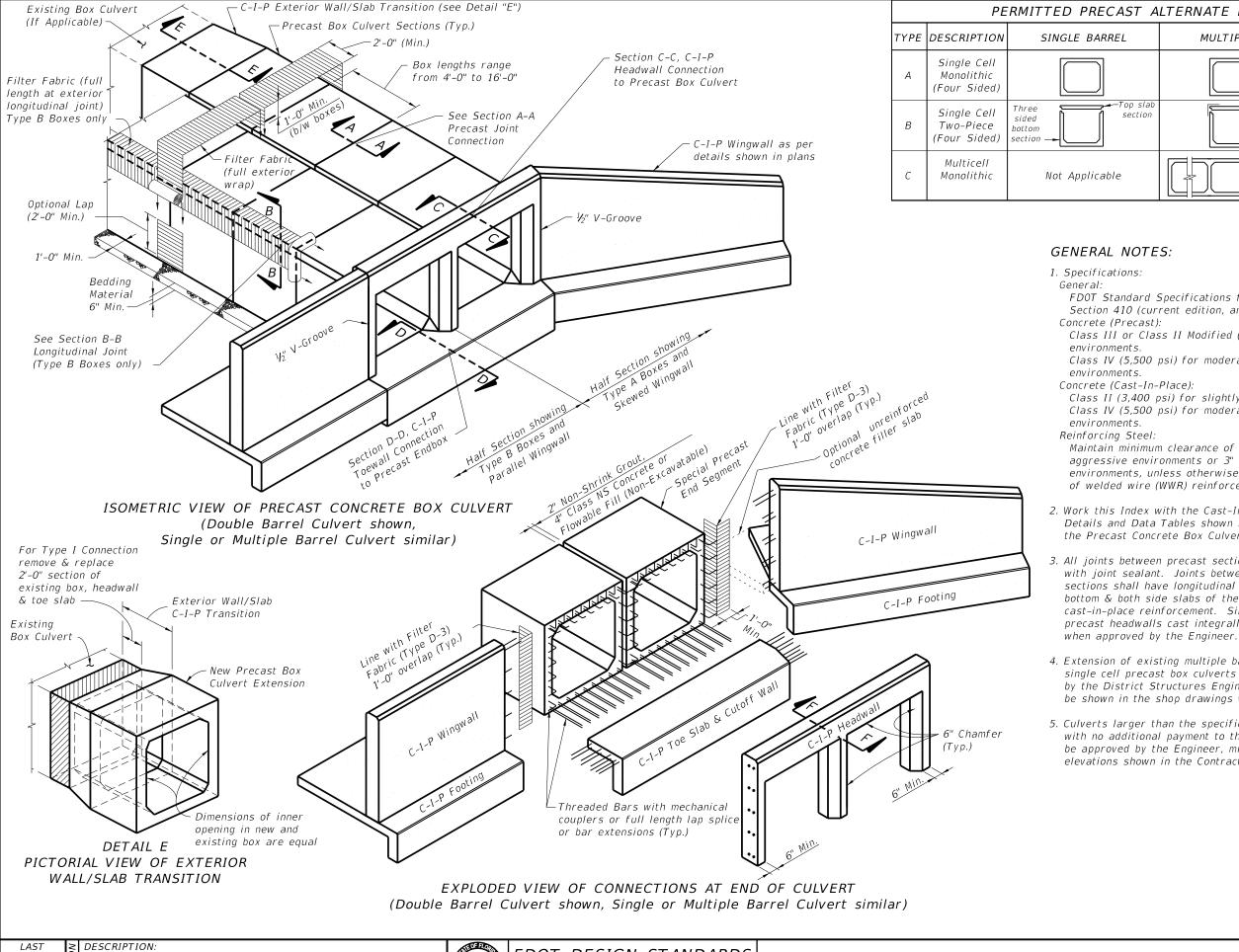






07/01/12





PERMITTED PRECAST ALTERNATE BOX SECTIONS MULTIPLE BARRELS DESIGN NOTES Index No. 292 Contractor Design Contractor Design Contractor Design

> FDOT Standard Specifications for Road and Bridge Construction, Section 410 (current edition, and supplements thereto).

> Class III or Class II Modified (5,000 psi) for slightly aggressive

Class IV (5,500 psi) for moderately to extremely aggressive

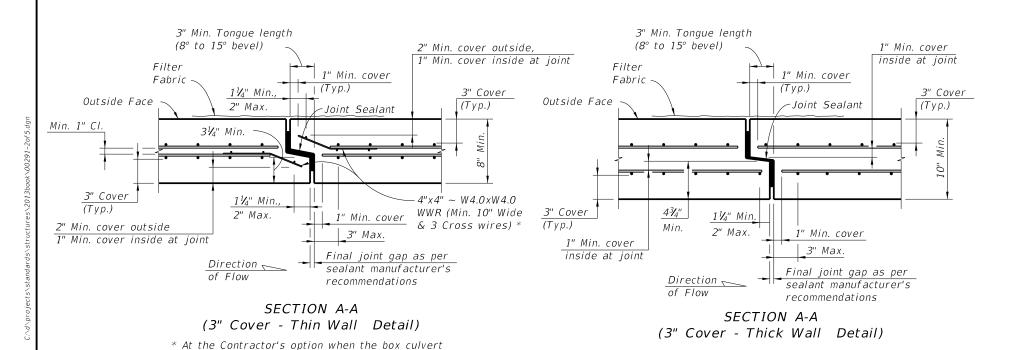
Class II (3,400 psi) for slightly aggressive environments. Class IV (5,500 psi) for moderately to extremely aggressive

Maintain minimum clearance of 2" for slightly and moderately aggressive environments or 3" for extremely aggressive environments, unless otherwise shown. Equal area substitution of welded wire (WWR) reinforcement is permitted.

- 2. Work this Index with the Cast-In-Place Concrete Box Culvert Details and Data Tables shown in the plans, Index No. 289 and the Precast Concrete Box Culverts shown in the shop drawings.
- 3. All joints between precast sections must be tongue & groove with joint sealant. Joints between cast-in-place & precast sections shall have longitudinal reinforcing extending from top, bottom & both side slabs of the precast box tied to the cast-in-place reinforcement. Single barrel culverts may have precast headwalls cast integrally with the end segment
- 4. Extension of existing multiple barrel box culverts with multiple single cell precast box culverts is not permitted unless approved by the District Structures Engineer. Full transition details must be shown in the shop drawings when approved.
- 5. Culverts larger than the specified size may be substituted with no additional payment to the Contractor. Substitution must be approved by the Engineer, minimum earth cover and invert elevations shown in the Contract Documents must be maintained.

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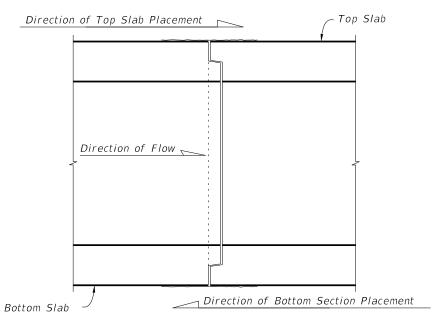
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See Section A-A 3" Min. Tongue length for reinforcing (8° to 15° bevel) cover requirements 3" Min. 3" Min. -Joint Sealant Provide WWR or extend reinforcing into tongue (See Section A-A)

ALTERNATE BOTTOM SLAB TRANSVERSE JOINT TYPICAL SECTION (DOUBLE-SIDED TONGUE & GROOVE JOINT) (All reinforcing not shown for clarity)

Bottom Slab Joints in Type B Boxes may be single tongue & groove joints as shown in Section A-A when the Top Slab Joints are oriented as shown in Schematic "A".



SCHEMATIC "A" TYPE B BOX SECTION PLACEMENT FOR SINGLE TONGUE & GROOVE JOINTS

= TWO-PIECE PRECAST SEGMENT = ADDITIONAL JOINT DETAILS (TYPE B BOX)

PRECAST SEGMENT TO SEGMENT TONGUE & GROOVE TRANSVERSE JOINTS =

reinforcing utilizes WWR, extend wall and slab

reinforcing into the joint and bend to maintain

of box to allow bending of the WWR.

cover in lieu of 4"x4" ~ W4.0xW4.0 WWR at joint.

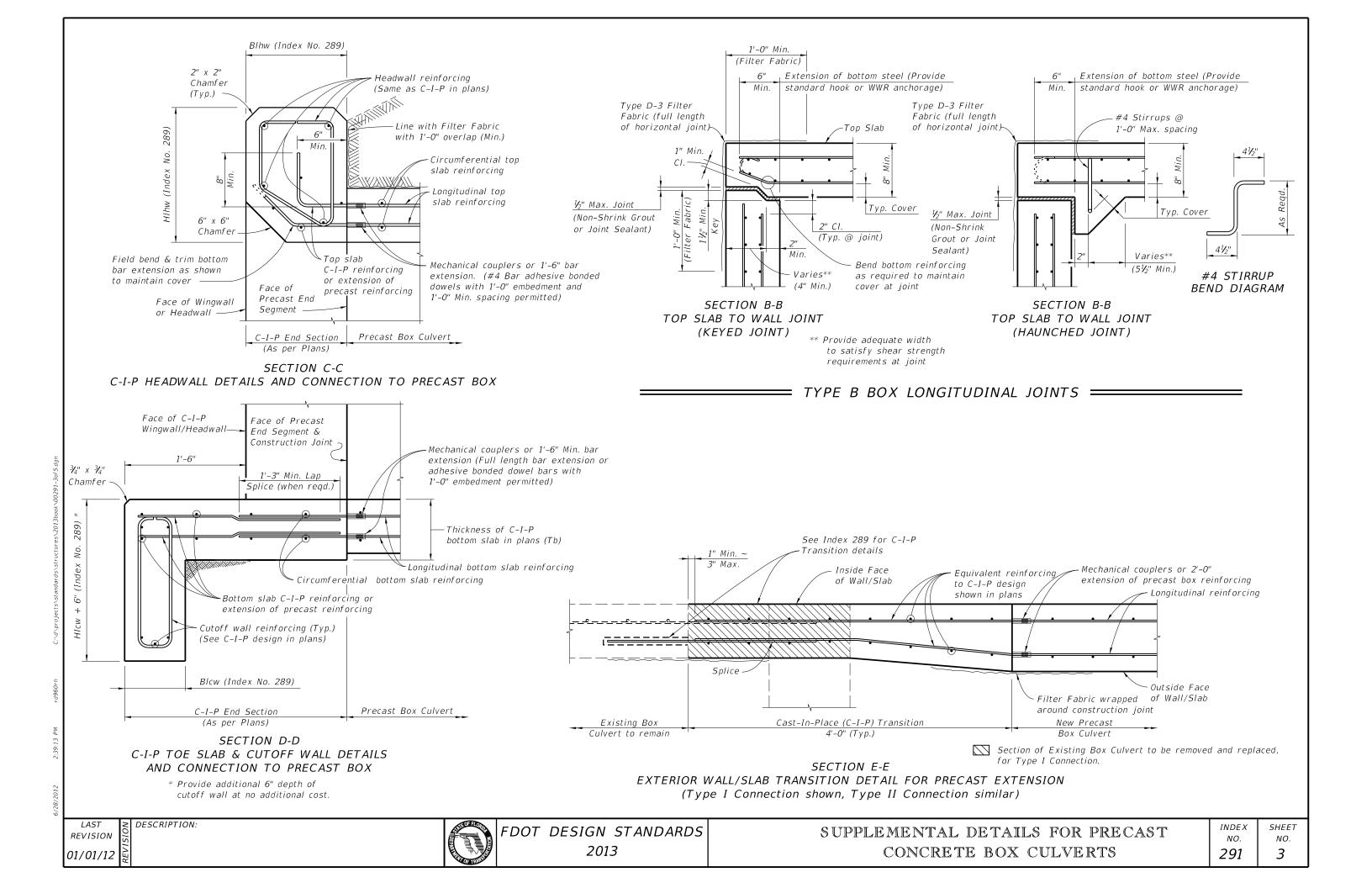
Transverse wire in tongue may be cut at corners

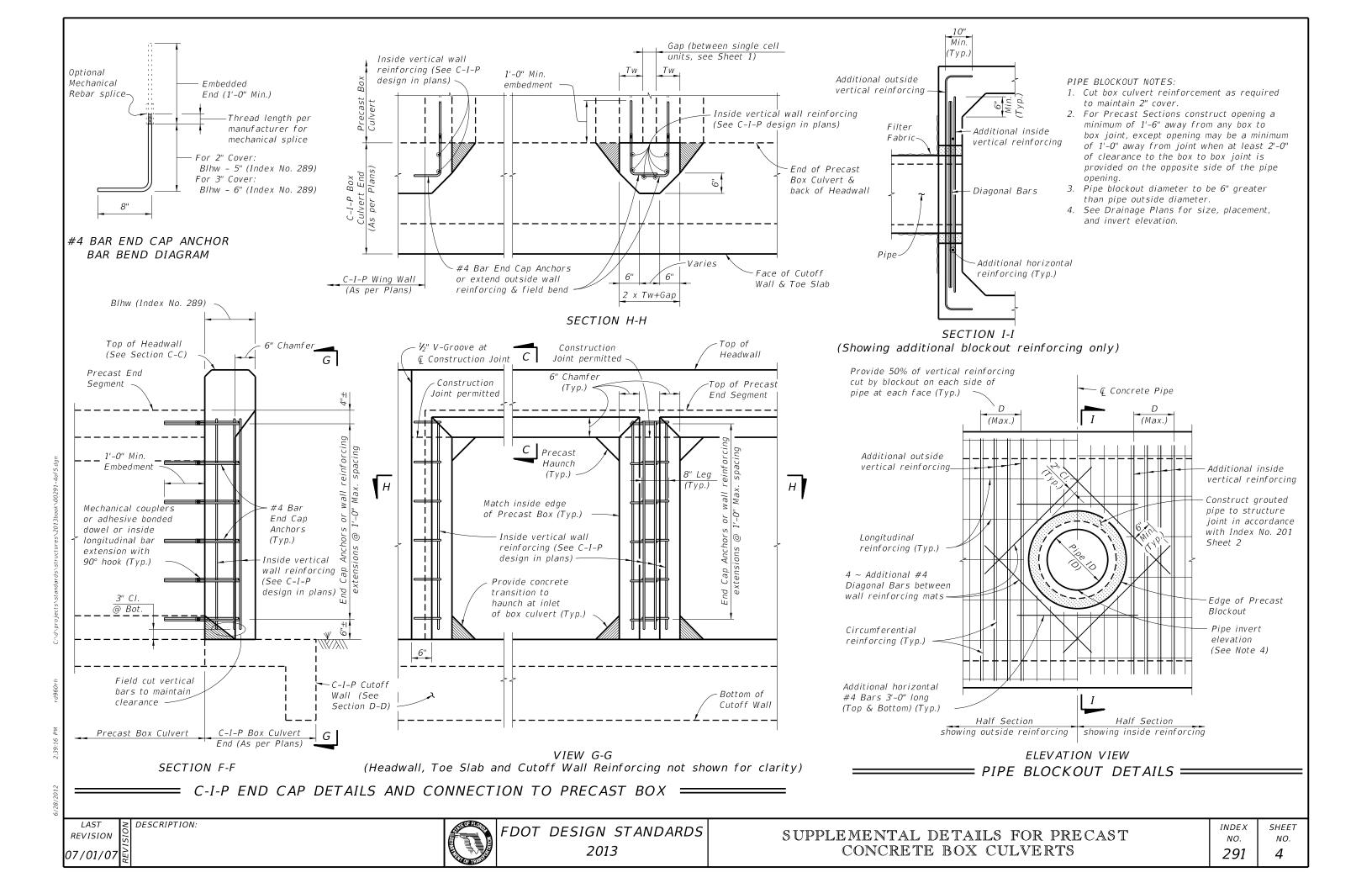
LAST REVISION 07/01/06

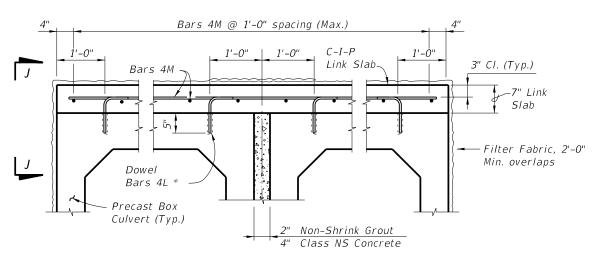
DESCRIPTION:



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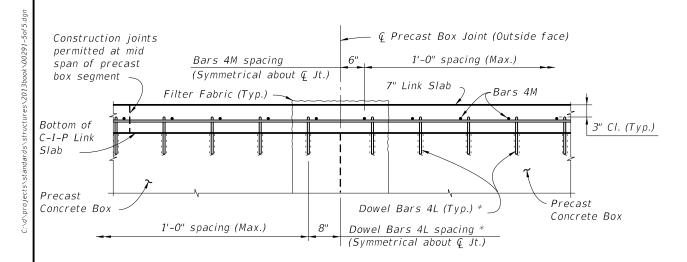






LINK SLAB TYPICAL SECTION (Multiple Barrel Culvert shown, Single Barrel Culvert similar)

* Install dowels with an Adhesive Bonding Material System in accordance with Specification Section 416. The Contractor may substitute mechanical couplers in lieu of adhesive bonded dowels. Shift dowels to clear box culvert reinforcing.



VIEW J-J

LINK SLAB NOTES:

1. Provide a Cast-In-Place Link Slab to ensure uniform joint opening of precast box culverts when the differential settlement shown in the plans exceeds the following limits, except that a Link Slab is not required for differential settlements less than 1/2".

$$\Delta Y \leq \frac{(L)^2}{760 \times R \times W}$$

Where:

 $\Delta Y = Maximum Long-Term Differential Settlement (ft.)$

R = Exterior height of Box Culvert (ft.)

W = Length of Box Culvert Segments (ft.)

L = Effective length for single curvature deflection (ft.)

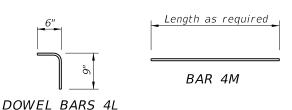
2. Extend Link Slab to back face of headwalls and to limits of existing box culverts for extensions.

ESTIMATED LINK SLA	B QUANTI	TIES
ITEM	UNIT	QUANTITY
Class II or IV Concrete (Culvert)	CY/SF	0.0216
Reinforcing Steel (Roadway)	Lb./SF	1.52

NOTE: Estimated quantities are based the plan area of precast box slabs, and are provided for information only. No additional payment will be made for Link Slabs where these are required for the precast box culverts.

	BILL OF RE	INFORCING STEE	L
MARK	SIZE	NO. REQ'D	LENGTH
L	4	2 per Barrel/Ft.	1'-3"
М	4	As Reqd.	As Reqd.

REINFORCING STEEL BENDING DIAGRAMS

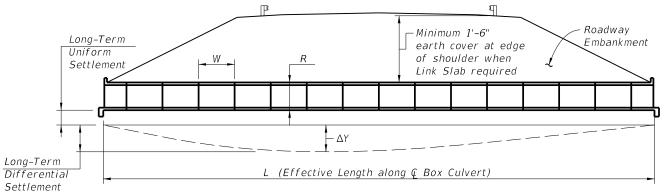


NOTES:

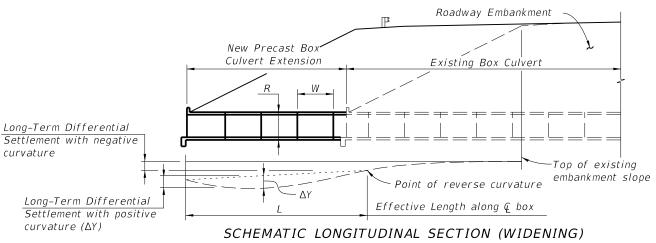
- 1. All bar dimensions are out to out.
- 2. Lap splice length for Bars 4M is 1'-4" minimum.

DESIGN NOTE:

1. Link Slab required when joint openings from differential settlement exceed 1/8" as determined in Link Slab Note 1.



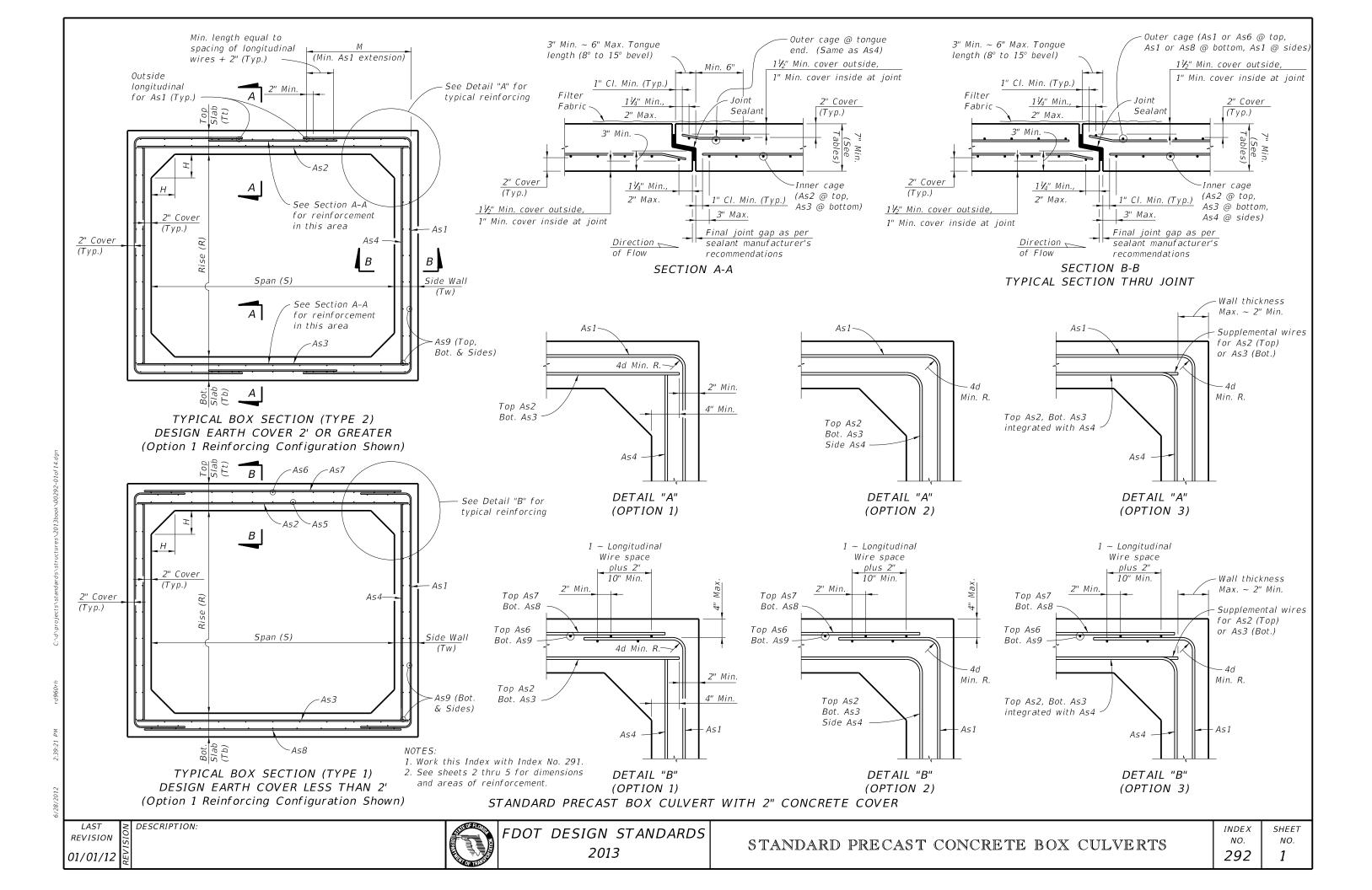
SCHEMATIC LONGITUDINAL SECTION (NEW CONSTRUCTION)



= DIFFERENTIAL SETTLEMENT COUNTERMEASURES FOR PRECAST BOX CULVERTS =

LAST REVISION

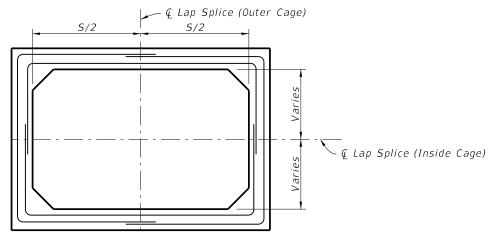




GENERAL NOTES:

- 1. These precast designs may be substituted for cast-in-place box culverts designed to AASHTO LRFD Bridge Design Specifications, 4th Edition. Designs are based on the design criteria shown in FDOT Structures Design Guidelines.
- 2. Loading: HL-93 & any fill heights between the minimum & maximum shown.
- 3. Only one design of precast box culvert is to be used for any installation.
- 4. Reinforcing steel must consist of smooth or deformed welded wire reinforcement (WWR) conforming to ASTM A185 or A497. Longitudinal reinforcement may consist of ASTM A615 Grade 60 bars. Minimum cover must be 2" unless otherwise shown. The spacing of circumferential wires must not be less than 2" nor more than 4". The spacing of longitudinal wires or bars must not be more than 8".
- 5. As9 longitudinal wires must have a minimum cross-sectional area of 40% of the circumferential wires, but not less than a W2.5 or D2.5 for WWR, or #3 bars for ASTM A615 deformed bars.
- 6. Welding of reinforcement must be limited to the locations shown in ASTM C1577 and in accordance with ANSI/AWS D1.4 "Structural Welding Code - Reinforcing Steel".
- 7. For alternate reinforcing configuration Options 2 and 3 shown in Detail "A" and "B" (Sheet 1), As1 may be extended to the middle of either slab and lap spliced with As7 and As8. As4 may be lap spliced at any location or connected to As2 or As3 at corners by welding.
- 8. Haunch dimensions may vary between the minimum and maximum dimensions shown in the Design Tables but only one haunch dimension must be used within the full length of the box culvert installation.

- 9. Submittal of redesign calculations are not required for any increase to the slab and/or wall thickness when the minimum reinforcement areas shown in the Design Tables are provided.
- 10. For Design Earth Cover greater than 10 feet, the Contractor may interpolate the required areas of reinforcement and slab or wall thickness. Interpolated areas of reinforcement, slab or wall thickness must be approved by the Engineer.
- 11. Minimum length of precast box segments is 4 feet and maximum length is 16 feet.
- 12. See Index No. 291 for connections to wingwalls, headwalls and other general details.



SCHEMATIC OF LAP SPLICE LOCATIONS FOR OPTION 2 & 3 REINFORCING CONFIGURATIONS

					CAST BOX	CULVE	RT D						& 4' S	SPANS		TAB
SPAN x RISE (S) (R)	TOP (Tt)	BOT. (Tb)		HAUNCH (H)	DESIGN EARTH COVER ABOVE			R		RCEMEN q.in./F	T AREA t.)	15			As1 EXT. LENGTH (M)	SPAN x RISE (S) (R)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As6	As7	As8	As9	(in.)	(Ft.)
					0.33' - <2'	0.17	0.29	0.21	0.17	0.17	0.17	0.17	0.17		-	
				4	2' - <3'	0.13	0.28	0.21	0.09	-	-	-	-		31	
					3' - <5'	0.09	0.17	0.17	0.09	-	-	-	-		31	
					5' - 10'	0.09	0.17	0.17	0.09	-	-	-	-		31	
3' x 3'	7	7	7	to	15'	0.09	0.17	0.17	0.09	-	-	-	-		31	3' x 3'
					20'	0.12	0.17	0.17	0.09	-	-	-	-		31	
					25'	0.14	0.18	0.18	0.09	-	-	-	-		31	
				8	30'	0.17	0.21	0.22	0.09	-	-	-	-		31	
					35'	0.19	0.25	0.25	0.09	-	-	-	-	5	31	
					0.33' - <2'	0.19	0.38	0.26	0.17	0.19	0.17	0.17	0.19		-	
				4	2' - <3'	0.19	0.38	0.26	0.09	-	-	-	-	Note	38	
				7	3' - <5'	0.14	0.20	0.22	0.09	-	-	-	-	1	38	
4' x 3'	7	7	7	to	5' - 10'	0.11	0.17	0.17	0.09	-	-	-	-	eneral	38	4' x 3'
4 1 2	′	′	′	"	15'	0.15	0.17	0.18	0.09	-	-	-	_	ene	38	7 ^ 3
				8	20'	0.20	0.23	0.23	0.09	-	-	-	-	9	38	
					25'	0.24	0.28	0.29	0.09	-	-	-	-	See	38	
					30'	0.29	0.34	0.35	0.09	-	-	-	-] ''	38	
					0.33' - <2'	0.19	0.41	0.28	0.17	0.21	0.17	0.17	0.19		-	
				4	2' - <3'	0.19	0.41	0.28	0.09	-	-	-	-		38	
				4	3' - <5'	0.14	0.21	0.24	0.09	-	-	-	-		38	
4' x 4'	7	7	7	to	5' - 10'	0.12	0.17	0.17	0.09	-	-	-	-		38	4' x 4'
4 / 4	′	′	′	"	15'	0.16	0.19	0.20	0.09	-	-	-	-		38	* ^ *
				8	20'	0.21	0.25	0.25	0.09	-	-	-	-	1	38	
				"	25'	0.26	0.31	0.32	0.09	-	-	-	-	1	38	
					30'	0.31	0.37	0.38	0.09	-	-	-	-	1	38	

TABI	LE 1B	- STA	ANDA	RD PRE	CAST BOX	CULVE	ERT D	ESIG	NS (2	" COV	/ER) -	- 3'	& 4' 5	SPANS	5
SPAN x RISE	SLAE	3 / WAL	L THIC	KNESS	DESIGN			R	EINFOR	CEMEN	T AREA	<i>S</i>			As1 EXT.
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)				LENGTH
(5+1)	(Tt)	(Tb)	(Tw)	(H)	ABOVE										(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As6	As7	As8	As9	(in.)
					0.33' - <2'	0.20	0.26	0.32	0.20	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.16	0.25	0.31	0.10	-	-	-	-		31
					3' - <5'	0.10	0.20	0.20	0.10	-	-	-	-		31
					5' - 10'	0.10	0.20	0.20	0.10	-	-	-	-		31
3' x 3'	8	8	8	to	15'	0.10	0.20	0.20	0.10	-	-	-	-		31
					20'	0.10	0.20	0.20	0.10	-	-	-	-		31
					25'	0.11	0.20	0.20	0.10	-	-	-	-		31
				8	30'	0.13	0.20	0.20	0.10	-	-	-	-		31
					<i>35</i> ′	0.15	0.21	0.21	0.10	-	-	-	-	5	31
					0.33' - <2'	0.20	0.31	0.22	0.20	0.20	0.20	0.20	0.20		-
				4	2' - <3'	0.12	0.31	0.22	0.10	-	-	-	-	Note	38
				,	3' - <5'	0.12	0.20	0.20	0.10	-	-	-	-		38
4' x 3'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	-	General	38
, , , ,					15'	0.12	0.20	0.20	0.10	-	-	-	-	en	38
				8	20'	0.16	0.20	0.20	0.10	-	-	-	-	1	38
					25'	0.19	0.24	0.24	0.10	-	-	-	-	See	38
					30'	0.22	0.28	0.29	0.10	-	-	-	-		38
					0.33' - <2'	0.20	0.33	0.24	0.20	0.20	0.20	0.20	0.20		_
				4	2' - <3'	0.17	0.33	0.24	0.10	-	-	-	-		38
				,	3' - <5'	0.12	0.20	0.20	0.10	-	-	-	-		38
4' x 4'	8	8	8	to	5' - 10'	0.10	0.20	0.20	0.10	-	-	-	-		38
, , ,					15'	0.13	0.20	0.20	0.10	-	-	-	-		38
				8	20'	0.16	0.21	0.22	0.10	-	-	-	-		38
					25'	0.20	0.26	0.27	0.10	-	-	-	-		38
					30'	0.23	0.31	0.32	0.10	-	-	-	-		38
	NOT	FS: 1	See St	neet 1 for	Reinforcina De	etails a	nd dim	ension	locatio	15					

NOTES: 1. See Sheet 1 for Reinforcing Details and dimension locations. 2. See Sheet 14 for WWR Bending Diagram.

LAST REVISION 01/01/12

TAB	LE 2A	- ST	ANDAI	RD PRE	CAST BOX	CULV	ERT L	DESIGN	IS (2"	COV	'ER)	- 5'	& 6'	SPAN:	S	TABL	.E 2B	- STA	ANDAF	RD PRE	CAST BOX	CULV	ERT D	ESIG	NS (2	e" COV	/ER) -	- 5' (& 6' S	SPANS	<u></u>
SPAN x RISE	SLAE	/ WAL	L THIC	KNESS	DESIGN			REI	INFORC			15			As1 EXT.	SPAN x RISE	SLAE	3 / WAL	THICK	KNESS	DESIGN			R	EINFOF	RCEMEN	T AREA.	.5			As1 EX
(S) (R)	TOP			HAUNCH					(sq.	in./Ft	.)				LENGTH	(S) (R)	TOP			HAUNCH					(5	q. in./Fi	t.)				LENGT
(Ft.)	(Tt)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB										(M) (in.)	(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB										(M) (in.)
(, (,,	(in.)	(111.)	(111.)	(111.)		As 1	A52			As5	A56	As7	As8	A59		(1 0.7	(111.)	(111.)	(111.)	(111.)		As1	A52	As 3		As5	A56	As7	A58	As9	
					0.33' - <2' 2' - <3'	0.31	0.48	l — l —	0.17	0.21	0.17	0.23	0.31	-	- 1E						0.33' - <2'	0.26 0.26	0.39	0.36 0.36		0.20	0.20	0.20	0.26		45
i I				4	3' - <5'	0.31	0.48	 	0.09	-	_	_		1	45 36					4	2' - <3' 3' - <5'	0.26	0.39		0.10	_		_			36
5' x 3'	7	7	7	to	5' - 10'	0.20	0.19		0.09			_		-	36	5' x 3'	8	8	8	t o	5' - 10'	0.13	0.20	0.24		_					36
	′	′	,	10	15'	0.24	0.25		0.09	_	_	_	_	1	35					to	15'	0.19	0.21	0.22		_					35
i				8	20'	0.32	0.33		0.09	-	_	-	_	1	35					8	20'	0.24	0.28	0.28	0.10	_	-	_	_		35
					25'	0.39	0.41		0.09	-	-	-	_		35					Ü	25'	0.30	0.34	0.35		-	-	-	-		35
					30'	0.47	0.50	0.50	0.09	-	-	-	ı		35						30'	0.36	0.41	0.41	0.10	-	-	-	-		35
					0.33' - <2'	0.30	0.51	0.45	0.17	0.23	0.17	0.21	0.30		-						0.33' - <2'	0.25	0.42	0.38	0.20	0.20	0.20	0.20	0.25		-
4				4	2' - <3'	0.30	0.51		0.09	-	-	-	-		45					4	2' - <3'	0.25	0.42	0.38	0.10	-	-				45
					3' - <5'	0.18	0.30		0.09	-	-	-	-		45						3' - <5'	0.16	0.25	0.25		-	-		=		45
5' x 4'	7	7	7	to	5' - 10'	0.17	0.21		0.09	-	-	-	-		36	5' x 4'	8	8	8	to	5' - 10'	0.13	0.20	0.20		-	-	-			36
				_	15'	0.24	0.27		0.09	-	-	-	-	-	35						15'	0.19	0.23		0.10	-	-	-			35
1				8	20' 25'	0.31	0.36		0.09	-	-	-	-	-	35 35					8	20' 25'	0.24	0.30 0.37	0.31	0.10	_	-	_	-		35 35
					30'	0.39	0.45		0.09	_ +			_	1	35						25° 30'	0.30	0.37	0.38		_					35
	+				0.33' - <2'	0.40	0.53			0.24	0.17	0.21	0.30	1	-						0.33' - <2'	0.25	0.43	0.41		0.20	0.20	0.20	0.25		- 33
				$ $ $_{A}$	2' - <3'	0.29	0.53		0.09	_	-	-	-	1	45					Δ	2' - <3'	0.25	0.44		0.20	-	-				45
					3' - <5'	0.19	0.31		0.09	_	_	-		1	45					7	3' - <5'	0.16	0.26	0.27	0.10	_	-	-			45
5' x 5'	7	7	7	to	5' - 10'	0.19	0.22		0.09	-	-	-	-		45	5' x 5'	8	8	8	to	5' - 10'	0.15	0.20	0.22		-	-	_	-		45
					15'	0.26	0.29		0.09	-	-	-	-	1	36						15'	0.20	0.25	0.26	0.10	-	-	-	-		36
				8	20'	0.34	0.39	0.40	0.09	-	-	-	1		35					8	20'	0.26	0.32	0.33	0.10	-	-	_	-		35
					25'	0.41	0.49		0.09	-	-	-	-		35						25'	0.32	0.40	0.41	0.10	-	-	_	-		35
					30'	0.49	0.59		0.09	-	-	-	_	1	35						30'	0.37	0.48	0.49		-	-	-	_		35
	7.5	7	7		0.33' - <2'	0.39	0.54			0.22	0.18	0.25	0.39	5	_						0.33' - <2'	0.32	0.47	0.41	0.20	0.20	0.20	0.25	0.32	5	
				4	2' - <3'	0.39	0.58		0.09	-	-	-	-	Note	43					4	2' - <3'	0.32	0.47	0.41	0.10	-	-	-	-	Note	43
	7	7	7		3' - <5' 5' - 10'	0.28	0.36		0.09	-	-	-	_	N N	39	61 11 21	8	8	,		3' - <5'	0.23	0.30	0.31	0.10	-		-			39
6' x 3'	′	′	/	to	3 - 10 15'	0.25	0.26		0.09	_				era	39 38	6' x 3'	0	"	8	to	5' - 10' 15'	0.19	0.22 0.29		0.10	_		_		neral	38
				12	20'	0.30	0.46		0.09			_		Gen	38					12	20'	0.26	0.29	0.29		_		_		Gen	38
	7	7.5	7	12	25'	0.59	0.57		0.09	-	_	_		6	38					1 2	25'	0.45	0.47	0.47	0.10	_	_				38
i	8	8	7		30'	0.60	0.64		0.09	-	_	-	_	Se	38						30'	0.54	0.57	0.57	0.10	_	-	_	_	See	38
1	7.5	7	7		0.33' - <2'	0.37	0.58	0.52	0.17	0.24	0.18	0.23	0.37		_						0.33' - <2'	0.31	0.50	0.44		0.21	0.20	0.23	0.31		_
				4	2' - <3'	0.37	0.61		0.09	-	-	-	-	1	43					4	2' - <3'	0.31	0.50	0.44		-	- 1	_	-		43
					3' - <5'	0.26	0.39	0.39	0.09	-	-	-	-		39						3' - <5'	0.23	0.32	0.34	0.10	_	-	-	_		39
6' x 4'	7	7	7	to	5' - 10'	0.24			0.09	-	-	-	-		39	6' x 4'	8	8	8	to	5' - 10'	0.19	0.24			-	-		-		39
					15'	0.35			0.09	-	-	-	-		38						15'	0.27	0.31	0.32		-	-		_		38
	<u> </u>	7.5	_	12	20'	_			0.09	-	-	-	-	_	38					12	20'		0.41		0.10	-	-	-			38
	/	7.5	7		25'	0.56			0.09	-	-	-	-	-	38						25'		0.51		0.10	_	-	-	-		38
—	7.5	8	7		30' 0.33' - <2'	0.58			0.09	0.25	0 10	0 22	0.36	1	38						30' 0.33' - <2'		0.62 0.52		0.10	0.22	0.20	-	0.30		38
	/.5	-'-		A	2' - <3'	0.36			0.17		0.18	U.ZZ	0.30	1	43					Δ	0.33° - <2° 2' - <3'	1	0.52			0.22		0.22	0.30		43
1				4	3' - <5'		0.410		0.09		_	_	_	1	43					4	3' - <5'					_	_				43
6' x 5'	7	7	7	to	5' - 10'	0.25			0.09	_ +	_	_	_	1	39	6' x 5'	8	8	8	to	5' - 10'		0.26		0.10	_	_		_		39
	'				15'		0.40		0.09	_	-	_	_	1	38		-		-		15'				0.10	_	_		_		38
	L			12	20'	0.46			0.09	-	-	-	-	1	38					12	20'		0.44		0.10	-	-	_	-		38
	7	7.5	7		25'	0.56	0.67		0.09	-	-	-	_		38						25'	0.44	0.55	0.55	0.10	_		-	-		38
	8	8	8		30'	0.60			0.09	-		-	-]	38						30'	0.52	0.66		0.10	-	-		-		38
	7.5	7	7		0.33' - <2'		0.63			0.26	0.18	0.22	.036	1	_						0.33' - <2'				0.20	0.22	0.20	0.22	0.30		_
				4	2' - <3'	0.35			0.09	-	-	-	-	4	52					4	2' - <3'	1	0.54		0.10	-	-	_	-		52
]	_	_			3' - <5'	0.27		t – t	0.09	-	-	-	-	1	52		_	_	_		3' - <5'		0.36	0.38		_	-		-		52
6' x 6'	7	7	7	to	5' - 10'	0.27			0.09	-	-	-	-	-	43	6' x 6'	8	8	8	to	5' - 10'	0.21	0.27		0.10	-	-		-		43
1					15'	0.38			0.09	-	-	-	-	-	39					4-	15'		0.35	0.37		-	-	-			39
	7	7.5	7	12	20'	0.50	1		0.09	-	-	-	-	-	39 38					12	20' 25'	0.38	0.47 0.59	0.48	0.10	-	-		-		39 38
. 1	<u>'</u>				25' 30'			0.70			-	_	_	-	38		1				30'	l	0.59	0.60		- 					38
	l 8	8	7		1 511										⊰× ■						1 31 <i>1</i>	1 11 77					_ '				

LAST DESCRIPTION:
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7	ABLE	3 - 3	STANI	DARD P	RECAST BO	X CU	LVERT	DES	IGNS	(2" (COVER	R) - 7	' SPA	N <i>S</i>	
SPAN x RISE (S) (R)	SLAB TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R		RCEMEN q. in./F		15			As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	A56	As7	A58	As9	(in.)
				4	0.33' - <2'	0.37	0.58	0.49	0.20	0.22	0.20	0.29	0.37		_
				4	2' - <3'	0.37	0.58	0.49	0.10	-	_	-	-		43
				to	3' - <5'	0.30	0.40	0.42	0.10	-	-	-	-		43
7' x 4'	8	8	8	"	5' - 10'	0.26	0.30	0.33	0.10	-	-	-	-		43
				12	15'	0.37	0.40	0.40	0.10	-	-	-	-		41
				12	20'	0.49	0.53	0.53	0.10	-	-	-	-		41
	8	8	8	7 to	25'	0.60	0.67	0.66	0.10	-	-	-	-		41
	8.5	8.5	8	12	30'	0.68	0.79	0.78	0.10	-	-	-	-		41
				4	0.33' - <2'	0.36	0.60	0.53	0.20	0.23	0.20	0.28	0.36		-
				4	2' - <3'	0.36	0.60	0.53	0.10	-	-	-	_		47
				to	3' - <5'	0.30	0.42	0.45	0.10	-	-	-	-		43
7' x 5'	8	8	8	1 10	5' - 10'	0.26	0.32	0.35	0.10	-	-	-	-		43
				12	15'	0.37	0.43	0.44	0.10	-	-	-	-	5	41
				12	20'	0.48	0.57	0.57	0.10	-	-	-	-	Note	41
	8	8	8	7 to	25'	0.60	0.72	0.72	0.10	-	-	-	-	NC	41
	8.5	8.5	8	12	30'	0.67	0.84	0.84	0.10	-	-	-	_	le.	41
				4	0.33' - <2'	0.36	0.63	0.56	0.20	0.24	0.20	0.27	0.36	General	-
				4	2' - <3'	0.36	0.63	0.56	0.10	-	-	-	-	<i>Ge</i>	59
				to	3' - <5'	0.29	0.44	0.47	0.10	-	-	-	-	өө	47
7' x 6'	8	8	8	1 10	5' - 10'	0.27	0.34	0.37	0.10	-	-	-	-	Sí	43
				12	15'	0.38	0.46	0.46	0.10	-	-	-	-		41
				12	20'	0.49	0.60	0.61	0.10	-	-	-	-		41
	8	8	8	7 to	<i>25</i> ′	0.61	0.76	0.76	0.10	-	-	-	_		41
	8.5	8.5	8	12	30'	0.69	0.89	0.89	0.10	-	-	-	-		41
				4	0.33' - <2'	0.36	0.65	0.58	0.20	0.25	0.20	0.27	0.36		-
				"	2' - <3'	0.36	0.65	0.58	0.10	-	-	-	-		59
				to	3' - <5'	0.30	0.46	0.50	0.10	-	-		-		59
7' x 7'	8	8	8	"	5' - 10'	0.30	0.35	0.50	0.10	-	-	-	-		47
				12	15'	0.41	0.48	0.50	0.10	-	-	-	-		43
				12	20'	0.53	0.64	0.65	0.10	-	-	-	-		43
	8	8	8	7 to	25'	0.65	0.80	0.81	0.10	-	-	-	-		43
	8.5	9	8	12	30'	0.72	0.92	0.91	0.10	-	1	-	-		41

	ABLE	4 - 9	STANI	DARD P	RECAST BO	X CU	LVERT	DES	IGNS	(2" C	OVE	R) - 8	' SPA	NS	,
SPAN x RISE			L THIC		DESIGN			R	EINFOF			15			As1 EXT.
(S) (R)	TOP	BOT.	1	HAUNCH					(5	q. in./F	t.)				LENGTH
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	ABOVE TOP SLAB										(M) (in.)
(1 2.)		_ `	<u> </u>	(111.)		As 1	As2	As3	As4	As5	A56	As7	A58	As9	(111.)
	9	8.5	8	4	0.33' - <2'	0.40	0.60	0.52	0.20	0.22	0.22	0.28	0.39		
					2' - <3'	0.45	0.66	0.54	0.10	-	-	-	-		50
0, 4,				to	3' - <5'	0.39	0.48	0.50	0.10	-	-	-	-		50
8' x 4'	8	8	8		5' - 10'	0.34	0.38	0.40	0.10	-	-	-	-		45
				12	15'	0.49	0.51	0.50	0.10	-	-	-	-		41
	0.5	0.5		0 1	20'	0.65	0.68	0.66	0.10	-	-	-	-		41
	8.5	8.5	8	8 to	25'	0.76	0.83	0.80	0.10	-	-	-	-		41
	9.5	9.5	8	12	30'	0.79	0.94	0.92	0.10	-	-	-	-		41
	9	8.5	8	4	0.33' - <2'	0.38	0.65	0.59	0.20	0.22	0.22	0.30	0.37		-
					2' - <3'	0.43	0.69	0.58	0.10	-	-	-	-		50
0, 5,	0	0		to	3' - <5'	0.37	0.51	0.53	0.10	-	-	-	-		45
8' x 5'	8	8	8		5' - 10'	0.33	0.41	0.42	0.10	-	-	-	-		45
				12	15'	0.48	0.54	0.53	0.10	-	-	-	-		41
	0.5	0.5		0 1-	20'	0.63	0.73	0.70	0.10	-	-	-	-		41
	8.5	8.5	8	8 to	25'	0.74	0.88	0.86	0.10	-	-	-	-		41
	9.5	9.5	8	12	30'	0.77	1.00	0.98	0.10	-	-	-	-		41
	9	9	8	4	0.33' - <2'	0.32	0.65	0.58	0.20	0.23	0.22	0.25	0.31	5 5	-
					2' - <3'	0.42	0.71	0.61	0.10	-	-	-	-	Note	50
0, 0	0	0		to	3' - <5'	0.37	0.54	0.56	0.10	-	-	-	-		50
8' x 6'	8	8	8		5' - 10'	0.34	0.43	0.45	0.10	-	-	-	-	ıra	45
				12	15'	0.49	0.57	0.57	0.10	-	-	-	-	General	41
-	0.5	0.5		0 4-	20' 25'	0.64	0.77	0.76	0.10	-	-	-	-		41
-	8.5	8.5	8	8 to		0.74	0.94	0.92	0.10	-	-	-	-	See	41
	9.5	9.5	8	12	30'	0.78	1.05	1.04	0.10	-	-	- 0.24	- 0.21	0,	41
	9	9	8	4	0.33' - <2'	0.31	0.67	0.60	0.20	0.24	0.22	0.24	0.31		-
					2' - <3'	0.42	0.74	0.64	0.10	-	-	-	-		55
01 71	0	0	8	to	3' - <5'	0.37	0.56	0.59	0.10	-	-	-	-		55
8' x 7'	8	8	"		5' - 10'	0.36	0.45	0.47	0.10		_	_	-		50
				12	15'	0.51	0.61	0.61	0.10	-	_	_	_		45
}	8.5	8.5	8	8 to	20' 25'	0.66 0.78	0.81	0.80 0.97	0.10	_	_	_	_		41
}	9.5	9.5	8	12	30'	0.78	1.10	1.09	0.10	_	_	_	_		41
	9.5	9.5	8	12											<u> </u>
}	9	9	⁸	4	0.33' - <2' 2' - <3'	0.32	0.68	0.62 0.67	0.20	0.24	0.22	0.25	0.32		65
						0.7.0			0.14	_	-	_	-		
QI V OI	8	8	8	to	3' - <5' 5' - 10'	0.38	0.58	0.61	0.14		_		_		65 55
8' x 8'	σ	8	°			0.39		0.50			_	_	_		
				12	15' 20'	0.55	0.64	0.65	0.10	-	-	-	-		45 45
-	0 =	0 5	0	8 to		0.71	0.86	0.85	0.10	_	_		_		
-	8.5	8.5 9.5	8 8		25' 30'	0.84	1.03	1.02		_	_	-	_		41
	9.5	9.5	ا ٥	12	30	0.93	1.15	1.15	0.10			_	_		41

- NOTES:
 1. See Sheet 1 for Reinforcing Details and dimension locations.
 2. See Sheet 2 for General Notes.

STANDARD PRECAST CONCRETE BOX CULVERTS

3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.

TABLE 5 - STANDARD PRECAST BOX CULVERT DESIGNS (2" COVER) - 9' SPANS

REINFORCEMENT AREAS

T.	ABLE	6 - 5	STANE	ARD P	RECAST BOX	x CUL	VERT	DES	IGNS	(2" C	OVER	?) - 10	' SPA	NS	
SPAN x RISE		/ WAL	L THIC		DESIGN			R	EINFOR			15			As1 EXT
(S) (R)	TOP	BOT.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)				LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE										(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As6	As7	As8	A59	(in.)
				4	0.33' - <2'	0.46	0.62	0.52	0.24	0.24	0.24	0.41	0.45		-
				7	2' - <3'	0.46	0.62	0.52	0.12	-	-	-	-		58
				to	3' - <5'	0.42	0.54	0.50	0.12	-	-	-	-		53
10' x 5'	10	10	10	10	5' - 10'	0.38	0.46	0.49	0.12	-	-	-	-		52
				12	15'	0.52	0.59	0.58	0.12	-	-	-	-		47
				12	20'	0.69	0.78	0.76	0.12	-	-	-	-		47
	10.5	10.5	10	8 to	25'	0.81	0.97	0.93	0.12	-	-	-	-		47
	11.5	12	10	12	30'	0.87	1.11	1.11	0.12	-	-	-	-		47
				4	0.33' - <2'	0.44	0.64	0.54	0.24	0.24	0.24	0.39	0.44		-
				7	2' - <3'	0.44	0.64	0.54	0.12	-	-	-	-		58
				to	3' - <5'	0.39	0.57	0.52	0.12	-	-	-	-		<i>52</i>
10' x 6'	10	10	10		5' - 10'	0.37	0.48	0.52	0.12	-	-	-	-		<i>52</i>
				12	15'	0.51	0.62	0.61	0.12	-	-	-	-		47
				12	20'	0.67	0.83	0.80	0.12	-	-	-	-		47
	10.5	10.5	10	8 to	25'	0.79	1.02	0.99	0.12	-	-	-	-		47
	11.5	12	10	12	30'	0.85	1.17	1.14	0.12	-	_	-	_		47
				4	0.33' - <2'	0.43	0.66	0.57	0.24	0.24	0.24	0.38	0.43		_
				4	2' - <3'	0.43	0.66	0.57	0.12	-	-	-	ı		58
				to	3' - <5'	0.38	0.59	0.55	0.12	-	-	-	-		58
10' x 7'	10	10	10	10	5' - 10'	0.37	0.50	0.54	0.12	-	-	-	-		52
				12	15'	0.52	0.66	0.65	0.12	-	-	-	-	5	47
				12	20'	0.67	0.87	0.85	0.12	-	-	-	1	ıte	47
	10.5	10.5	10	8 to	25'	0.79	1.07	1.04	0.12	-	-	-	ı	Not	47
	11.5	12	10	12	30'	0.84	1.22	1.19	0.12	-	-	-	-	General	47
				4	0.33' - <2'	0.43	0.68	0.60	0.24	0.24	0.24	0.38	0.43	neı	-
				4	2' - <3'	0.43	0.68	0.60	0.12	-	-	-	-	<i>Ge</i>	64
				to	3' - <5'	0.38	0.62	0.57	0.12	-	-	-	-	өө	58
10' x 8'	10	10	10	10	5' - 10'	0.38	0.52	0.57	0.12	-	-	-	-	Sí	52
				12	15'	0.53	0.69	0.68	0.12	-	-	-	-		47
				12	20'	0.68	0.91	0.89	0.12	-	-	-	-		47
	10.5	10.5	10	8 to	25'	0.81	1.12	1.09	0.12	-	-	-	-		47
	11.5	12	10	12	30'	0.86	1.27	1.25	0.12	-	-	-	ı		47
				4	0.33' - <2'	0.43	0.70	0.62	0.24	0.24	0.24	0.38	0.43		-
				7	2' - <3'	0.43	0.70	0.62	0.12	-	-	-	-		70
				to	3' - <5'	0.39	0.64	0.60	0.12	-	-	-	-		64
10' x 9'	10	10	10	"	5' - 10'	0.40	0.54	0.59	0.12	-	-	-	-		58
				12	15'	0.56	0.72	0.72	0.12	-	-	-	-		52
				12	20'	0.71	0.95	0.94	0.12	-	-	-	-		47
	10.5	11	10	8 to	25'	0.82	1.15	1.13	0.12	-	-	-	-		47
	11.5	12	10	12	30'	0.90	1.32	1.30	0.12	-	-	-	-		47
				4	0.33' - <2'	0.44	0.71	0.64	0.24	0.24	0.24	0.38	0.44		-
				"	2' - <3'	0.44	0.71	0.64	0.17	-	-	-	-		79
				to	3' - <5'	0.40	0.65	0.62	0.16	-	-	-	-		70
10' × 10'	10	10	10	"	5' - 10'	0.44	0.56	0.61	0.15	_	_	_	_		64
				12	15'	0.60	0.75	0.76	0.12	-	-	-	-		52
				12	20'	0.76	0.99	0.99	0.12	-	-	-	-		52
ļ	10.5	11	10	8 to	25'	0.86	1.20	1.18	0.12	-	-	-	-		47
	11.5	12	10	12	30'	0.97	1.36	1.35	0.13	-	_	-	-		47

NOTES:

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for WWR Bending Diagram.

LAST REVISION 07/01/07



SPAN x RISE

(Ft.)

 $11' \times 4'$

11' x 6'

11' x 8'

11' × 10'

11' × 11'

(R)

TOP

(Tt)

(in.)

11

13

11

11.5

13

11

11.5

13

11

11.5

13

11

SLAB / WALL THICKNESS

(Tb)

(in.)

11

11.5 11.5 11

11

11.5

13

11

11

12

13.5

11

12

13.5

11.5 11

13 11

13 11

BOT. | SIDE | HAUNCH |

(H)

(in.)

12

8 to

12

12

8 to

12

12

8 to

12

8 to

12

8 to

12

(Tw)

(in.)

11

11

11

11

11

11

11

11

11

11

11

LAST REVISION 01/01/07	REVISION	

TABLE 7 - STANDARD PRECAST BOX CULVERT DESIGNS (2" COVER) - 11' SPANS

As1 | As2 |

0.57

0.50

0.58

0.77

0.95

1.09

0.62

0.62

0.58

0.56

0.65

0.86

1.07

1.22

0.63

0.60

0.72

0.94

1.16

1.32

0.71

0.71

0.67

0.64

0.78

1.03

1.24

1.40

0.72

0.69

0.66

0.81

1.07

1.29

0.47

0.47

0.46

0.50

0.56

0.74

0.91

1.06

0.52

0.52

0.51

0.56

0.64

0.83

1.03

1.19

0.57

0.57

0.56

0.61

0.71

0.92

1.13

1.30

0.62

0.62

0.61

0.66

0.78

1.01

1.22

1.39

0.64

0.64

0.63

0.69

0.82

1.06

1.27

1.44 1.44

0.27

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.27

0.14

0.14

0.14

0.14

0.14

0.14

0.14

0.27

0.18

0.18

0.16

0.14

0.14

0.14

0.14

0.51

0.48

0.47

0.59

0.77

0.92

0.94

0.45

0.45

0.42

0.43

0.54

0.70

0.83

0.85

0.42

0.43

0.39

0.43

0.54

0.70

0.82

0.86

0.44

0.44

0.41

0.47

0.59

0.75

0.85

0.91

0.45

0.45

0.42

0.51

0.63

0.80

0.91

0.99

DESIGN EARTH COVER

ABOVE

TOP SLAB

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15' 20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

REINFORCEMENT AREAS

(sq. in./Ft.)

0.27 | 0.27 | 0.27 |

0.27 | 0.27 | 0.27

0.27

0.27 | 0.27

0.27

As3 | As4 | As5 | As6 | As7 | As8 | As9

0.45

0.41

0.39

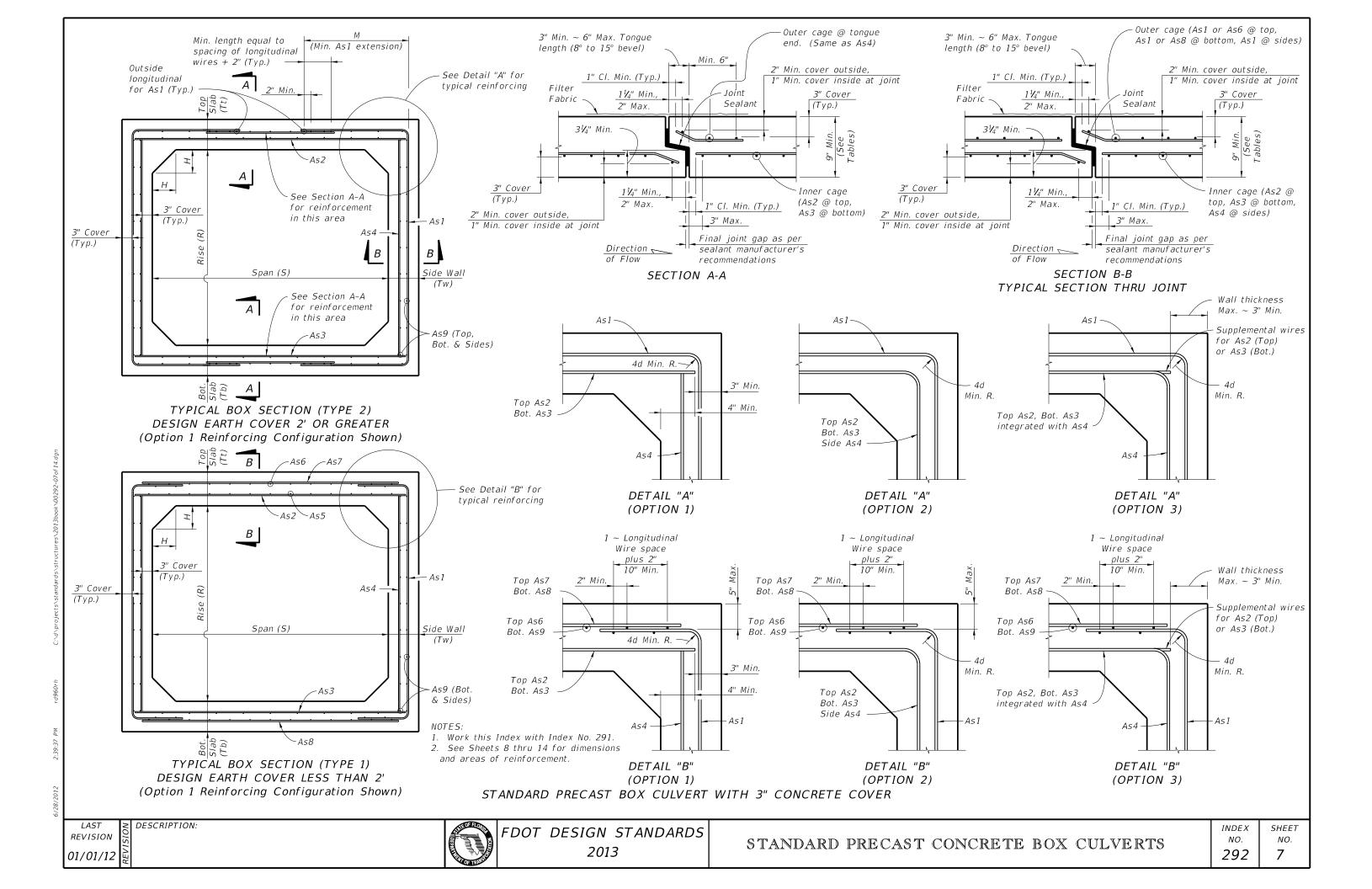
0.39

0.27 | 0.27 |

		T.	ABLE	8 - 5	STAND	DARD P	RECAST BO	X CUL	VERT	DES	IGNS	(2" C	OVER	!) - 12	" SPA	NS	
As1 EX		SPAN x RISE	SLAB	 	L THIC		DESIGN			R			T AREA	15			As1 EXT.
LENGT F	′	(S) (R)	TOP (Tt)	BOT. (Tb)	SIDE (Tw)	HAUNCH (H)	EARTH COVER ABOVE				(5	q. in./F	t.)				LENGTH (M)
(in.)		(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As6	As7	As8	A59	(in.)
_	-						0.33' - <2'	0.52	0.57	0.45	0.29	0.29	0.29	0.47	0.49	ASS	_
62						4	2' - <3'	0.52	0.57	0.45	0.15	-	-	-	-		73
62						to	3' - <5'	0.50	0.54	0.45	0.15	-	-	-	-		66
55		12' x 4'	12	12	12	10	5' - 10'	0.50	0.52	0.52	0.15	-	-	-	-		66
55						12	15'	0.63	0.61	0.59	0.15	-	-	-	-		59
55							20'	0.82	0.81	0.77	0.15	-	-	-	-		59
55			12.5	12.5	12	8 to	25'	0.99	0.99	0.95	0.15	-	-	-	-		59
55			14	14	12	12	30'	1.03	1.15	1.11	0.15	-	- 0.20	- 12	- 16		59
- 62						4	0.33' - <2' 2' - <3'	0.47	0.62 0.62	0.51	0.29	0.29	0.29	0.42	0.46		-
62 55							2 - <3 3' - <5'	0.47 0.45	0.60	0.51 0.51	0.15	_	_	_	_		66 59
55		12' x 6'	12	12	12	to	5' - 10'	0.47	0.59	0.51	0.15		_	_			59
50		12 / 0	12	12	12	1.0	15'	0.57	0.68	0.66	0.15	_	_	_	_		53
50						12	20'	0.74	0.90	0.86	0.15	-	-	-	_		53
50			12.5	12.5	12	8 to	25'	0.88	1.11	1.06	0.15	-	-	-	-		53
50			14	14.5	12	12	30'	0.92	1.27	1.24	0.15	-	-	-	-		53
_						4	0.33' - <2'	0.44	0.67	0.56	0.29	0.29	0.29	0.40	0.44	5	_
62						7	2' - <3'	0.44	0.67	0.56	0.15	-	-	-	-	Note	66
62						to	3' - <5'	0.41	0.64	0.56	0.15	-	-		-		59
55		12' x 8'	12	12	12		5' - 10'	0.45	0.63	0.64	0.15	-	-	-	-	General	59
50						12	15'	0.56	0.75	0.73	0.15	-	-	-	-	aue	53
50	_		12.5	1.3	1.2	8 to	20'	0.72	0.98	0.95	0.15	-	-	-	-		53
50 50			12.5 14	14.5	12 12	12	25' 30'	0.85	1.20 1.38	1.16 1.35	0.15	-	-	-	_	See	53 53
-	-		14	14.5	12		0.33' - <2'	0.89	0.71	0.60	0.13	0.29	0.29	0.39	0.44	,	-
75						4	2' - <3'	0.44	0.71	0.60	0.25	-	-	-	-		73
69						,	3' - <5'	0.42	0.68	0.60	0.15	_	_	_	_		66
62		12' x 10'	12	12	12	to	5' - 10'	0.47	0.67	0.69	0.15	_	-	-	-		59
55						12	15'	0.59	0.81	0.81	0.15	-	-	-	-		53
50						12	20'	0.75	1.06	1.04	0.15	-	-	-	-		53
50			12.5	13	12	8 to	25'	0.87	1.30	1.26	0.15	-	_	-	-		53
50			14	14.5	12	12	30'	0.92	1.47	1.45	0.15	-	-	-	-		53
_						4	0.33' - <2'	0.46	0.74	0.64	0.29	0.29	0.29	0.40	0.46		-
86							2' - <3'	0.46	0.74	0.64	0.20	-	-	-	-		93
75						to	3' - <5'	0.42	0.72	0.64	0.20	-	-	-	-		80
69	-	12' x 12'	12	12	12		5' - 10'	0.54	0.71	0.74	0.18	-	-	-	-		73
55						12	15'	0.66	0.87	0.89	0.15	-	-	-	-		59
55 50			12.5	13	12	8 to	20' 25'	0.83	1.14	1.13 1.37	0.15	-	_	-	-		59 53
50	-		12.5	14.5	12.5	12	30'	1.05	1.56	1.56	0.15	_	_	_	_		53
1 50			17	17.5	12.5	1 12	1 20	1.03	1.50	1.50	1 0.13		_	_	_		رر

NOTES:

- 1. See Sheet 1 for Reinforcing Details and dimension locations.
- 2. See Sheet 2 for General Notes.
- 3. See Sheet 14 for Welded Wire Reinforcement Bending Diagram.



1	A TOP TO THE OWNER OF THE OWNER OWNER OF THE OWNER	
	TO STATE OF THE PARTY OF THE PA	

TAB	LE 9A	\ - ST	ANDA	ARD PRI	ECAST BOX	CULV	'ERT	DESIC	GNS (.	3" CO	VER)	- 3' 8	\$ 4' S	SPANS	5
SPAN x RISE (S) (R)	TOP (Tt)	B / WAL BOT. (Tb)	SIDE (Tw)	HAUNCH (H)	DESIGN EARTH COVER ABOVE			R		RCEMEN q. in./F		15			As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As1	As2	As3	As4	As5	As6	As7	As8	As9	(in.)
					0.33' - <2'	0.22	0.24	0.22	0.22	0.22	0.22	0.22	0.22		-
					2' - <3'	0.11	0.23	0.22	0.11	-	-	-	-		31
				4	3' - <5'	0.11	0.22	0.22	0.11	-	-	-	-		31
					5' - 10'	0.11	0.22	0.22	0.11	-	-	-	-		31
3' x 3'	9	9	9	to	15'	0.11	0.22	0.22	0.11	-	-	-	-		31
					20'	0.13	0.22	0.22	0.11	-	-	-	-		31
				8	25'	0.16	0.22	0.22	0.11	-	-	-	-		31
					30'	0.19	0.24	0.25	0.11	-	-	-	-		31
					35'	0.22	0.28	0.29	0.11	-	-	-	-	31	
					0.33' - <2'	0.22	0.32	0.24	0.22	0.22	0.22	0.22	0.22	5	_
				4	2' - <3'	0.17	0.31	0.24	0.11	-	-	-	-	Note	38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-	-		38
4' x 3'	9	9	9	to	5' - 10'	0.13	0.22	0.22	0.11	-	-	-	-	General	38
					15'	0.17	0.22	0.22	0.11	-	-	-	-	nei	38
				8	20'	0.23	0.26	0.27	0.11	-	-	-	-	<i>Ge</i>	38
					25'	0.28	0.32	0.34	0.11	-	-	-	-	өө	38
					30'	0.33	0.39	0.40	0.11	-	-	-	-	S	38
					0.33' - <2'	0.22	0.34	0.26	022	0.22	0.22	0.22	0.22		-
				4	2' - <3'	0.17	0.33	0.26	0.11	-	-	-	-		38
					3' - <5'	0.13	0.22	0.22	0.11	-	-	-	-		38
4' x 4'	9	9	9	to	5' - 10'	0.14	0.22	0.22	0.11	-	-	-	-		38
					15'	0.19	0.22	0.23	0.11	-	-	-	-		38
				8	20'	0.24	0.28	0.30	0.11	-	-	-	-		38
					25'	0.29	0.36	0.37	0.11	-	-	-	1		38
					30'	0.34	0.43	0.45	0.11	-	-	_	_		38

TABI	LE 9B	R - ST	ANDA	RD PRI	ECAST BOX	CULV	ERT	DESIG	GNS (.	3" CO	VER)	- 3' 8	§ 4' S	PANS	5
SPAN x RISE (S) (R)	SLAB TOP (Tt)	B / WAL BOT. (Tb)		KNESS HAUNCH (H)	DESIGN EARTH COVER ABOVE			R	EINFOR (s	RCEMEN q. in./F		15			As1 EXT. LENGTH (M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	As6	As7	As8	As9	(in.)
					0.33' - <2'	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24		_
					2' - <3'	0.12	0.24	0.24	0.24	-	-	-	-		31
				4	3' - <5'	0.12	0.24	0.24	0.24	-	-	-	-		31
					5' - 10'	0.12	0.24	0.24	0.24	-	-	-	-		31
3' x 3'	10	10	10	to	15'	0.12	0.24	0.24	0.24	-	-	-	-		31
					20'	0.12	0.24	0.24	0.24	-	-	-	-		31
				8	25'	0.13	0.24	0.24	0.24	-	-	-	-		31
					30'	0.15	0.24	0.24	0.12	-	-	-	-		31
					35'	0.18	0.24	0.24	0.12	-	-	-	-		31
					0.33' - <2'	0.24	0.26	0.24	0.24	0.24	0.24	0.24	0.24	5	_
				4	2' - <3'	0.14	0.26	0.24	0.12	-	-	-	-	Note	38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-	-	NC	38
4' x 3'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-	-	General	38
					15'	0.14	0.24	0.24	0.12	-	-	-	-	neı	38
				8	20'	0.18	0.24	0.24	0.12	-	-	-	_	<i>6e</i>	38
					25'	0.22	0.26	0.27	0.12	-	-	-	-	өө	38
					30'	0.26	0.31	0.32	0.12	1	1	-	ı	S	38
					0.33' - <2'	0.24	0.28	0.24	0.24	0.24	0.24	0.24	0.24		-
				4	2' - <3'	0.14	0.28	0.24	0.12	-	-	-	-		38
					3' - <5'	0.12	0.24	0.24	0.12	-	-	-	-		38
4' x 4'	10	10	10	to	5' - 10'	0.12	0.24	0.24	0.12	-	-	-	_		38
					15'	0.15	0.24	0.24	0.12	-	-	-	-		38
				8	20'	0.19	0.24	0.24	0.12	-	1	-	1		38
					25'	0.23	0.28	0.30	0.12	-	1	-	1		38
					30'	0.27	0.34	0.35	0.12	-	-	-	-		38

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

					ECAST BOX	CUL	/ EKI						& b′.	SPAN.							ECAST BOX	CULV	'EKI						α σ΄ .	SPAN	_
SPAN x RISE (S) (R)	SLAB TOP			KNESS HAUNCH	DESIGN EARTH COVER			REIN		MENT in./Ft.)		5			As1 EXT. LENGTH	SPAN x RISE (S) (R)	SLAB TOP	/ WAL BOT.	LTHICK	KNESS HAUNCH	DESIGN EARTH COVER			R		RCEMENT q. in./Ft		5			As1 EX
(10)	(Tt)	(Tb)	(Tw)	(H)	ABOVE				(39.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					(M)	(10)	(Tt)	(Tb)	(Tw)	(H)	ABOVE				(5)	9. 111.,1	/				(M)
(Ft.)	(in.)		(in.)	(in.)	TOP SLAB	As 1	As2	As3 As	4 /	NS5 /	A56	As7	As8	A59	(in.)	(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	A54	As5	A56	As7	A 5 8	As9	(in.)
					0.33' - <2'	0.27	0.39	0.37 0.2				0.22	0.27	7,33	_						0.33' - <2'	0.24	0.33	0.32	0.24	0.24	0.24	0.24	0.24	ASS	-
'				4	2' - <3'	0.26	0.39	0.37 0		-	-	-	-	1	45					Δ	2' - <3'	0.22	0.33	0.32	0.12	-	-	-	-	1	45
'				'	3' - <5'	0.19	0.24	0.25 0.		-	-	_	_		36					,	3' - <5'	0.16	0.24	0.24		-	-	_	_	1	36
5' x 3'	9	9	9	to	5' - 10'	0.20	0.22	0.22 0	_	-	-	_	-	1	36	5' x 3'	10	10	10	to	5' - 10'	0.16	0.24	0.24		-	-	-	_	1	36
'				"	15'	0.28	0.28	0.30 0.		-	-	_	_	1	35						15'	0.23	0.24	0.24		-	-	-	_	1	35
'				8	20'	0.37	0.38	0.39 0	1 1	-	-	_	-		35					12	20'	0.29	0.30	0.31	0.12	-	-	-	-	1	35
'					25'	0.45	0.48	0.49 0	1 1	-	-	-	-	1	35						25'	0.36	0.38	0.39	0.12	-	-	-	-	1	35
'					30'	0.54	0.58	0.59 0.	1 1	-	-	-	-	1	35						30'	0.43	0.46	0.47	0.12	-	-	-	-	1	35
'					0.33' - <2'	0.26	0.42	0.39 0.2	22 0	.22 (0.22	0.22	0.26		-						0.33' - <2'	0.24	0.35	0.34	0.24	0.24	0.24	0.24	0.24		_
'				4	2' - <3'	0.26	0.42	0.39 0.	1 1	-	-	_	-		45					4	2' - <3'	0.22	0.35	0.34	0.12	-	-	-	-		45
'					3' - <5'	0.19	0.26	0.27 0	11	-	-	-	-		45						3' - <5'	0.15	0.24	0.24	0.12	-	-	-	-		45
5' x 4'	9	9	9	to	5' - 10'	0.20	0.22	0.23 0	11	-	-	-	-		36	5' x 4'	10	10	10	to	5' - 10'	0.16	0.24	0.24	0.12	-	-	-	-		36
'					15'	0.27	0.31	0.33 0		-	-	-	-		35						15'	0.22	0.25	0.27	0.12	-	-	-	-		35
'				8	20'	0.36	0.42	0.43 0		-	-	_	-	_	35					12	20'	0.29	0.33	0.34		-	-	-	-	1	35
'					25'	0.44	0.52	0.54 0.		-	-	-	-	4	35						25'	0.36	0.41	0.43		-	-	-	-	1	35
 '					30'	0.53	0.63	0.65 0.		-	-	-	-	4	35						30'	0.42	0.50	0.51	0.12	-	-	-	-	-	35
'					0.33' - <2'	0.27	0.44	0.42 0.2	_	.22 (0.22	0.22	0.27	4	- 45						0.33' - <2'	0.24	0.37	0.36		0.24	0.24	0.24	0.24	-	-
'				4	2' - <3'	0.27	0.44	0.42 0		-	-	-	-	4	45					4	2' - <3'	0.21	0.37	0.36		-	-	-	-	-	45
l					3' - <5'	0.20	0.27	0.28 0		-	-		-	-	45	E, E,	1.0	10	10		3' - <5'	0.16	0.24	0.25		-	-	_	-	-	45
5' x 5'	9	9	9	to	5' - 10'	0.22	0.23	0.26 0		-	-		_	-	45	5' x 5'	10	10	10	to	5' - 10'	0.17	0.24	0.24		-	-	-	-	-	45
'				_	15'	0.30	0.34	0.36 0		-	-		-	-	36						15'	0.24	0.27	0.29		-	-	-	_	-	36
'				8	20' 25'	0.38	0.45	0.47 0		-	-	_	_	-	35 35					12	20' 25'	0.30	0.36 0.44	0.38	0.12	-	-		-	-	35 35
'					30'	0.47	0.56	0.59 0		-	-		_	-	35						30'	0.37	0.44	0.47	0.12	-		_	_	-	35
					0.33' - <2'	0.33				.22 (2 2 2	0.25	0.24	2	-						0.33' - <2'	0.44	0.40	0.36		0.24	0.24	0.24	0.28	5	
'					2' - <3'	0.34		0.42 0.2		.22	_	0.23	- 0.34	ا س	43					Δ	2' - <3'	0.28	0.40	0.36		0.24	0.24	0.24	0.20	1 0.	43
'				4	3' - <5'	0.27	0.31	0.32 0	_	-	_			Not	39					4	3' - <5'	0.22	0.40	0.28	0.12	_				Not	39
6' x 3'	9	9	9	to	5' - 10'	0.29	0.26	0.28 0.		_	_	_	_	<u></u>	39	6' x 3'	10	10	10	to	5' - 10'	0.24	0.24	0.24		_	_	_			39
				10	15'	0.42	0.39	0.40 0.		-	_	_	_	ler.	38		.		10	10	15'	0.34	0.31	0.32		_	_	_	_	General	38
'				12	20'	0.55	1	0.53 0.		-	-	_	_	Ger	38					12	20'	0.44	0.41	0.42		-	-	_	-	Ser	38
'				1 12	25'	0.68	0.66	0.67 0	1 1	-	-	_	-	J	38						25'	0.54	0.52	0.53	0.12	-	-	-	-	l o	38
'					30'	0.82	0.81	0.82 0	1 1	-	-	-	-	Se	38						30'	0.64	0.63	0.64	0.12	-	-	-	-	Se	38
,					0.33' - <2'	0.33	0.50	0.46 0.2	22 0	.22 (0.22	0.23	0.33	1	-						0.33' - <2'	0.27	0.42	0.39	0.24	0.24	0.24	0.24	0.27	1	_
'				4	2' - <3'	0.33	0.50	0.46 0.	1 1	-	-	-	-		43					4	2' - <3'	0.27	0.42	0.39	0.12	-	-	-	ı		43
'					3' - <5'	0.27	0.33	0.35 0	1 1	-	-	_	-		39						3' - <5'	0.21	0.28	0.30	0.12	-	-	-	-		39
6' x 4'	9	9	9	to	5' - 10'	0.28	1	0.31 0	1 1	-	-	-	-		39	6' x 4'	10	10	10	to	5' - 10'	1	0.24	0.25	0.12	-	-	-	-		39
'					15'	0.40			_	-	-	-	-		38						15'	0.32	0.34		0.12	-	-	-	-		38
'				12	20'	0.52		0.59 0.		-	-	-	-	4	38					12	20'		0.45			-	-	-	-	4	38
'					25'	0.65		0.74 0		-	-	-	_	4	38						25'	0.51	0.56			-	-	-	_	4	38
'					30'	0.78		0.90 0.		-	-		-	4	38						30'		0.68	0.70		-	-	-	-	4	38
'					0.33' - <2'	0.33				.22 0	0.22	0.23	0.33	-	- 42						0.33' - <2'		0.44			0.24	0.24	0.24	0.26	-	- 42
'				4	2' - <3'	0.33		0.49 0		-	-		-	4	43					4	2' - <3'	0.26	0.44			-	-	-	-	-	43
C FI					3' - <5'	0.27		0.37 0	_	-	-	-	-	-	43	CL v. Fl	10	10	1.0		3' - <5'	0.22	0.30			-	-	_	_	-	43
6' x 5'	9	9	9	to	5' - 10'	0.29		0.34 0	_	-	-			-	39 38	6' x 5'	10	10	10	to	5' - 10' 15'		0.25			-	-			-	39
'				1.2	15'	0.41		0.49 0.		-	_			-	38					1.0						_				1	38
				12	20' 25'	0.53		0.64 0.			_			1	38					12	20' 25'	0.42	0.48	0.51 0.63						1	38
, 					30'	0.78		0.80 0		_	_ +		_	1	38						30'	0.52	0.74			_	_		_	1	38
·'					0.33' - <2'	0.34				.22 (2.22	0.24	0.34	1	-						0.33' - <2'	0.27	0.46		0.24	0.24	0.24	0 24	0.27	1	
١,				1	2' - <3'	0.34	•	0.51 0.2		_	_	-	-	1	52					4	2' - <3'	0.27					-	-	-	1	52
l 	1			"	3' - <5'	0.29		0.39 0.		_	_	_	_	1	52					4	3' - <5'		0.31			_	_	_	_	1	52
ļ			1	1	5' - 10'	0.32		0.37 0		_	_	_	-	1	43	6' x 6'	10	10	10	to	5' - 10'		0.27			_	-	-	-	1	43
6' x 6'	9	9	9	to	J - 10									_																4	· -
6' x 6'	9	9	9	to	15'	0.44			_	-	-	-	-		39						15'	0.35	0.39	0.42	0.12	-	-	-	-		39
6' x 6'	9	9	9		-		0.50	0.53 0	11	-	-	-	-	-	39 39					12	15' 20'	0.35	0.39 0.52			-	-	-	-		39 39
6' x 6'	9	9	9	12	15'	0.44	0.50	0.53 0	1 1 1 1	- - -		- - -	- - -	_						12	15' 20' 25'			0.42 0.55 0.68	0.12	- - -	- - -		- -	-	

LAST REVISION 01/01/07

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TAST REVISION 7/01/07 A DESCRIPTION: FDOT DESIGN STANDARDS 2013						
		ION	DESCRIPTION:	FDOT	DESIGN STAI	VDARDS
7/01/0/		Ι,				
	//01/0/	RE			2013	

TABLE 11A - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 7' SPANS

As1 As2

0.58

0.41

0.40

0.56

0.76

0.97

1.18

0.61

0.61

0.43

0.41

0.61

0.82

1.04

1.26

0.63

0.45

0.44

0.65

0.87

1.11

1.35

0.66

0.65

0.47

0.47

0.69

0.93

1.18

1.43 1.36

0.52

0.51

0.44

0.39

0.58

0.77

0.97

1.10

0.55

0.55

0.47

0.43

0.63

0.83

1.06

1.19

0.58

0.58

0.50

0.47

0.68

0.90

1.13

1.27

0.61

0.61

0.52

0.52

0.74

0.97

1.22

0.22

0.11

0.11

0.11

0.11

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0.11

0.11

0.11

0.11

0.42

0.42

0.36

0.39

0.56

0.74

0.92

1.09

0.41

0.41

0.37

0.39

0.56

0.73

0.90

1.06

0.42

0.42

0.38

0.41

0.57

0.75

0.93

1.07

0.44

0.44

0.41

0.44

0.62

0.80

0.99

1.12

DESIGN

EARTH COVER

ABOVE

TOP SLAB

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15' 20'

25'

30'

REINFORCEMENT AREAS

(sq. in./Ft.)

0.22 | 0.23 | 0.22

0.22 | 0.24 | 0.22

0.25

0.22

As3 | As4 | As5 | As6 | As7 | As8 | As9

0.31

0.30

0.30

0.31

0.22 | 0.22 |

As1 EXT

LENGTH

(M)

(in.)

43

43

43

41

41

41

41

47

43

43 41

41

41

41

59

47

43

41

41

41

41

59

59

47

43

43

43

41

SLAB / WALL THICKNESS

(Tb)

(in.)

9.5

9.5

9.5

9.5

BOT. | SIDE | HAUNCH |

(H)

(in.)

(Tw)

(in.)

9

9

9

9

9

9

9 | 7 <u>to 12</u>

12

7 to 12

12

12

7 to 12

9 | 7 to 12

SPAN x RISE

(Ft.)

7' x 4'

7' x 5'

7' x 6'

7' x 7'

(R)

TOP

(Tt)

(in.)

T,	ABLE	11B -	STAN	DARD I	PRECAST BC	X CL	ILVER	T DE.	SIGNS	5 (3"	COVE	R) - 7	r' SPA	NS	
SPAN x RISE (S) (R)	TOP	BOT.	SIDE	HAUNCH	DESIGN EARTH COVER ABOVE			R	EINFOR (s	CEMEN q. in./F		15			As1 EXT. LENGTH (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	As 1	As2	As3	As4	As5	As6	As7	As8	A59	(in.)
					0.33' - <2'	0.33	0.49	0.44	0.24	0.24	0.24	0.24	0.33		-
				4	2' - <3'	0.33	0.49	0.44	0.12	-	-	-	-		43
					3' - <5'	0.29	0.35	0.38	0.12	ı	1	-	1		43
7' x 4'	10	10	10	to	5' - 10'	0.31	0.30	0.31	0.12	1	-	-	1		43
					15'	0.44	0.44	0.45	0.12	-	-	-	-		41
				12	20'	0.58	0.59	0.60	0.12	-	-	-	-		41
					25'	0.71	0.74	0.75	0.12	-	-	-	-		41
					30'	0.85	0.91	0.91	0.12	ı	-	-	ı		41
					0.33' - <2'	0.32	0.51	0.47	0.24	0.24	0.24	0.24	0.32		-
				4	2' - <3'	0.32	0.51	0.47	0.12	-	-	-	1		47
					3' - <5'	0.29	0.37	0.41	0.12	1	-	-	1		43
7' x 5'	10	10	10	to	5' - 10'	0.31	0.32	0.35	0.12	-	-	-	1		43
					15'	0.44	0.47	0.50	0.12	-	-	-	1	5	41
				12	20'	0.57	0.63	0.65	0.12	-	-	-	1	Note	41
					25 [']	0.70	0.80	0.82	0.12	-	-	-	-		41
					30'	0.84	0.97	0.99	0.12	-	-	-	-	General	41
					0.33' - <2'	0.33	0.53	0.50	0.24	0.24	0.24	0.24	0.33	ner	_
				4	2' - <3'	0.33	0.53	0.50	0.12	-	-	-	-	ge.	59
					3' - <5'	0.30	0.38	0.43	0.12	-	-	-	-	See	47
7' x 6'	10	10	10	to	5' - 10'	0.33	0.35	0.38	0.12	-	-	-	-	56	43
					15'	0.45	0.51	0.54	0.12	-	-	-	-		41
				12	20'	0.58	0.68	0.70	0.12	-	-	-	-		41
					25'	0.72	0.85	0.88	0.12	-	-	-	-		41
					30'	0.85	1.04	1.06	0.12	-	-	-	-		41
					0.33' - <2'	0.35	0.55	0.52	0.24	0.24	0.24	0.24	0.35		-
				4	2' - <3'	0.35	0.55	0.52	0.12	-	-	-	-		59
					3' - <5'	0.32	0.40	0.46	0.12	-	-	-	-		59
7' x 7'	10	10	10	to	5' - 10'	0.35	0.37	0.41	0.12	-	-	-	-		47
					15'	0.48	0.54	0.58	0.12	-	-	-	-		43
				12	20'	0.62	0.72	0.76	0.12	-	-	-	-		43
					25'	0.76	0.90	0.94	0.12	-	-	-	-		43
					30'	0.90	1.10	1.13	0.12	_	-	-	-		41

NOTES:

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

TO THE OWNER OF THE OWNER OWNER OF THE OWNER OWNE

(Ft.) (Tt) (in.) (/ WAL BOT. (Tb) (in.) 9 9.5 10.5		KNESS HAUNCH (H) (in.) 4 to	DESIGN EARTH COVER ABOVE TOP SLAB 0.33' - <2' 2' - <3' 3' - <5' 5' - 10'	As1 0.52 0.52	As2 0.66	As3	EINFOR (s	RCEMEN q. in./F		15			As1 EXT
8' x 4' 9 9 10 8' x 5' 9 9 10 8' x 6' 9 10 8' x 7' 9	9	9	4 to	0.33' - <2' 2' - <3' 3' - <5'	0.52		As3							LENGTH
8' x 5' 9 9 10 2 8' x 5' 9 9 10 2 8' x 6' 9 10 2 8' x 7' 9	9.5	_	to	2' - <3' 3' - <5'		0.66		As4	As5	As6	As7	As8	As9	(in.)
8' x 5' 9 9 10 2 8' x 6' 9 10 2 8' x 7' 9 9 10 2	9.5	_		3' - <5'	0.52		0.57	0.22	0.24	0.22	0.42	0.52		-
8' x 5' 9 9 10 2 8' x 6' 9 10 2 8' x 7' 9 9 10 2	9.5	_				0.66	0.57	0.11	-	-	-	-		50
8' x 5' 9 9 10 2 8' x 6' 9 10 2 8' x 7' 9 9 10 2	9.5	_	12		0.48	0.49	0.52	0.11	-	-	-	-		50
8' x 5' 9 9 10 8' x 6' 9 10 8' x 7' 9		9	12	15'	0.52 0.75	0.48	0.49	0.11	-		-	-		45
8' x 5' 9 9 10 8' x 6' 9 10 8' x 7' 9		9		20'	1.00	0.72	0.72	0.11	_			_		41
8' x 5' 9 9 10 8' x 6' 9 10 8' x 7' 9		9	8 to	25'	1.25	1.24	1.14	0.11	_	_	_	_		41
8' x 5' 9 9 10 8' x 6' 9 10 8' x 7' 9	10.5	9	12	30'	1.31	1.29	1.21	0.11	_			_		41
9 10 3 8' x 6' 9 9 10 8' x 7' 9				0.33' - <2'	0.51	0.69	0.60	0.22	0.25	0.22	0.40	0.51		
9 10 8' x 6' 9 9 10 8' x 7' 9			4	2' - <3'	0.51	0.69	0.60	0.11	-	-	-	-		50
9 10 3 8' x 6' 9 9 10 8' x 7' 9				3' - <5'	0.46	0.52	0.56	0.11	_	_	_	_		45
8' x 6' 9 9 10 2 8' x 7' 9	9	9	to	5' - 10'	0.51	0.51	0.53	0.11	_	_	_	_		45
8' x 6' 9 9 10 2 8' x 7' 9				15'	0.74	0.77	0.78	0.11	_	_	_	_		41
8' x 6' 9 9 10 2 8' x 7' 9			12	20'	0.97	1.05	1.05	0.11	-	_	-	-		41
8' x 6' 9 9 10 8' x 7' 9	9.5	9	8 to	25'	1.20	1.33	1.23	0.11	-	-	-	-		41
8' x 7' 9	10.5	9	12	30'	1.26	1.38	1.30	0.11	-	-	-	-		41
8' x 7' 9				0.33' - <2'	0.51	0.72	0.64	0.22	0.26	0.22	0.39	0.51	5	_
8' x 7' 9			4	2' - <3'	0.51	0.72	0.64	0.11	-	-	-	-	te	50
8' x 7' 9				3' - <5'	0.47	0.55	0.59	0.11	-	-	-	-	Note	50
8' x 7' 9	9	9	to	5' - 10'	0.52	0.55	0.58	0.11	-	-	-	-	le-	45
8' x 7' 9			12	15'	0.74	0.83	0.85	0.11	-	-	-	-	General	41
8' x 7' 9			12	20'	0.97	1.12	1.13	0.11	-	-	-	-		41
8' x 7' 9	9.5	9	8 to	25'	1.18	1.42	1.32	0.11	ı	1	-	-	See	41
9	10.5	9	12	30'	1.26	1.46	1.39	0.11	-	-	-	-	S	41
9			4	0.33' - <2'	0.52	0.74	0.67	0.22	0.26	0.22	0.40	0.52		_
9			7	2' - <3'	0.52	0.74	0.67	0.11	-	-	-	-		55
9			to	3' - <5'	0.49	0.57	0.62	0.11	-	-	-	-		55
	9	9		5' - 10'	0.55	0.59	0.63	0.11	-	_	-	-		50
			12	15'	0.77	0.88	0.91	0.11	-	-	-	-		41
		-		20'	1.01	1.19	1.21	0.11	-	-	-	-		41
10	9.5	9	8 to	25'	1.21	1.51	1.41	0.11	-	-	-	-		41
	10.5	9	12	30'	1.31	1.53	1.47	0.11	-	-	-	-		41
			4	0.33' - <2'	0.55	0.77	0.70	0.22	0.27	0.22	0.41	0.55		-
				2' - <3'	0.55	0.77	0.70	0.13	-	-	-	-		65
			to	3' - <5'	0.53	0.59	0.64	0.12	-	-	-	-		65
8' x 8' 9	_	9		5' - 10'	0.60	0.63	0.68	0.11	-	-	-	-		55
	9		12	15'	0.83	0.93	0.98	0.11	-	-	-	-		45
	9	0	0.40	20'	1.08	1.26	1.29	0.11	-	-	-	-		45
9 10	9.5	9	8 to 12	25' 30'	1.28 1.41	1.59 1.61	1.50 1.55	0.11	-	-	-	-		41

TA	ABLE	12B -	STAN	IDARD	PRECAST BO	οχ сι	JLVER	T DE	SIGNS	5 <i>(3</i> "	COVE	R) - (8' SPA	4 <i>NS</i>	
SPAN x RISE	SLAE	/ WAL	L THIC	KNESS	DESIGN			R	EINFOF			15			As1 EXT.
(S) (R)	TOP	вот.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)				LENGTH
	(Tt)	(Tb)	(Tw)	(H)	AB0VE										(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	As3	As4	As5	A56	As7	As8	A59	(in.)
					0.33' - <2'	0.42	0.56	0.49	0.24	0.24	0.24	0.32	0.41		-
				4	2' - <3'	0.42	0.56	0.49	0.12	-	-	-	-		50
					3' - <5'	0.38	0.42	0.46	0.12	-	-	-	-		50
8' x 4'	10	10	10	to	5' - 10'	0.41	0.38	0.39	0.12	-	-	-	-		45
					15'	0.59	0.56	0.57	0.12	-	-	-	-		41
				12	20'	0.78	0.75	0.76	0.12	-	-	-	-		41
					25'	0.97	0.96	0.96	0.12	-	-	-	-		41
	10	10.5	10	8 to 12	30'	1.15	1.16	1.10	0.12	-	-	-	-		41
					0.33' - <2'	0.40	0.58	0.52	0.24	.034	0.24	0.31	0.40		_
				4	2' - <3'	0.40	0.58	0.52	0.12	-	-	-	-		50
					3' - <5'	0.37	0.45	0.48	0.12	-	-	-	-		45
8' x 5'	10	10	10	to	5' - 10'	0.41	0.41	0.43	0.12	-	-	-	-		45
					15'	0.58	0.60	0.62	0.12	-	-	-	-		41
				12	20'	0.76	0.81	0.81	0.12	-	-	-	-		41
					25'	0.94	1.03	1.03	0.12	-	-	-	-		41
	10	10.5	10	8 to 12	30'	1.10	1.24	1.24	0.12	-	-	-	-		41
					0.33' - <2'	0.40	0.60	0.55	0.24	0.24	0.24	0.30	0.40	5	
				4	2' - <3'	0.40	0.60	0.55	0.12	-	-	-	-	Note	50
					3' - <5'	0.37	0.47	0.51	0.12	-	-	-	-		50
8' x 6'	10	10	10	to	5' - 10'	0.42	0.43	0.46	0.12	-	-	-	-	General	45
					15'	0.58	0.64	0.67	0.12	-	-	-	-	ene	41
				12	20'	0.76	0.86	0.88	0.12	-	-	-	-		41
					25'	0.94	1.09	1.11	0.12	-	-	-	-	See	41
	10	10.5	10	8 to 12	30'	1.09	1.32	1.26	0.12	-	-	-	-	0,	41
					0.33' - <2'	0.41	0.63	0.58	0.24	0.24	0.24	0.30	0.41		-
				4	2' - <3'	0.41	0.63	0.58	0.12	-	-	-	-		55
01 71	1.0	1.0	1.0		3' - <5'	0.39	0.49	0.53	0.12	-	-	-	-		55
8' x 7'	10	10	10	to	5' - 10' 15'	0.44	0.46	0.50	0.12	-	-	-	-		50
				4.0		0.61	0.68	0.72	0.12	_	_	_	_		45
				12	20' 25'	0.78	0.91 1.16	0.94 1.18	0.12	-	_	-	_		41
	10	10.5	10	0 to 12	30'	1.11	1.40	1.18	0.12	_	_	_			41
	10	10.5	10	8 to 12	0.33' - <2'			0.60	0.12	- 0.24		0.21	0.44		41
					0.33' - <2' 2' - <3'	0.44	0.64	0.60	0.24	0.24	0.24	0.31	-		65
				4	2' - <3' 3' - <5'	0.44	0.64	0.56	0.12		_	_	-		65
8' x 8'	10	10	10		5' - 10'	0.42	0.51	0.55	0.12	-	_	_	_		55
0 10	10	10	10	to	3 - 10 15'	0.47	0.30	0.33	0.12	_	_	_	_		45
				12	20'	0.84	0.72	1.01	0.12	_	_	_	_		45
				12	25'	1.03	1.22	1.26	0.12	_	_	_	_		41
	10	10.5	10	8 to 12	30'	1.16	1.47	1.42	0.12	_	_	_	_		41
	10	10.5	10	0 10 12	50	1.10	1.47	1.4∠	0.12						_ + 1

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
 See Sheet 14 for WWR Bending Diagrams.

FDOT DESIGN STANDARDS 2013

SPAN x RISE | SLAB / WALL THICKNESS

BOT.

(Tb)

(in.)

10.5

11.5

9

10.5

11.5

10.5

11.5

9.5

9

9.5

10.5

11.5

9.5

9

9.5

10.5

11.5

SIDE HAUNCH

(H)

(in.)

to

12

12

to

12

12

to

12

to

12

to

12

(Tw)

(in.)

9

9

9

9

9

9

9

9

9

9

9

9

9

9

9.5

TOP

(Tt)

(in.)

11

10

10

(Ft.)

9' x 5'

9' x 6'

9' x 7'

9' x 8'

9' x 9'

DESCRIPTION:

FDOT	L

TABLE 13A - STANDARD PRECAST BOX CULVERT DESIGNS (3" COVER) - 9' SPANS

0.62

0.62

0.58

0.65

0.95

1.26

1.39

1.46

0.60

0.60

0.56

0.65

0.94

1.25

1.37

1.44

0.61

0.61

0.58

0.67

0.96

1.27

1.38

1.49

0.60

0.64

0.62

0.71

1.01

1.27

1.45

1.59

0.68

0.68

0.68

0.79

1.37

1.56

1.56

0.78

0.78

0.63

0.63

0.96

1.32

1.41

1.50

0.81

0.81

0.66

0.68

1.03

1.40

1.49

1.58

0.84

0.83

0.69

0.73

1.09

1.49

1.57

1.70

0.85

0.86

0.72

0.77

1.16

1.56

1.65

1.72

0.88

0.88

0.75

0.82

1.22

1.64

1.73

1.73 1.68

0.65

0.65

0.61

0.64

0.95

1.28

1.32

1.42

0.69

0.69

0.65

0.69

1.02

1.38

1.40

1.50

0.72

0.72

0.68

0.75

1.10

1.47

1.48

1.58

0.73

0.76

0.72

0.81

1.17

1.45

1.57

1.66

0.76

0.78

0.78

0.88

1.26

1.54

1.65

0.22

0.11

0.11

0.11

0.11

0.11

0.11

0.22

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.11

0.22

0.12

0.11

0.11

0.11

0.11

0.11

0.11

0.22

0.18

0.18

0.17

0.13

0.13

0.13

0.12

0.29

0.29

0.22 | 0.28 |

DESIGN EARTH COVER

ABOVE

TOP SLAB

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

0.33' - <2'

2' - <3'

3' - <5'

5' - 10'

15'

20'

25'

30'

REINFORCEMENT AREAS

(sq. in./Ft.)

As1 | As2 | As3 | As4 | As5 | As6 | As7 | As8 | As9

0.26

0.27

0.22

0.27

0.22

0.22

0.22

0.55

0.52

0.51

0.51

		Γ	TA	ABLE	13B -	STAN	IDARD	PRECAST BO	OX CL	JLVER	T DE	SIGNS	5 (3"	COVE	R) - :	9' SP/	4 <i>NS</i>	
1	As1 EXT.	S	SPAN x RISE	SLAE	3 / WAL			DESIGN			R	EINFOF	RCEMEN	T AREA	15			As1 EXT
	LENGTH	((S) (R)	TOP	BOT.			EARTH COVER				(5	q. in./F	t.)				LENGTH
4	(M) (in.)		(Ft.)	(Tt)	(Tb)	(TW)	(H)	ABOVE TOP SLAB				T						(M) (in.)
4	(111.)	⊢	(1 (.)	(in.)	(in.)	(in.)	(in.)		As1	As2	As3	As4	As5	As6	As7	A58	A59	(111.)
ŀ	-						4	0.33' - <2'	0.49	0.65	0.57	0.24	0.24	0.24	0.40	0.48		
ŀ	54						7	2' - <3'	0.49	0.65	0.57	0.12	-	-	-	-		54
-	49			10	10	10	to	3' - <5'	0.46	0.54	0.53	0.12	-	-	-	-		49
-	49		9' x 5'	10	10	10		5' - 10'	0.52	0.50	0.51	0.12	-	-	-	-		49
-	44						12	15'	0.75	0.74	0.75	0.12	-	-	-		44	
ŀ	44							20'	0.98	1.01	1.00	0.12	-	-	-	-	_	44
┝	44			10	10.5	10	8 to	25'	1.21	1.27	1.19	0.12	-	_	-	-		44
┝	44	⊢		11	11.5	10	12	30'	1.30	1.36	1.30	0.12	-	-	-	-		44
┝	-						4	0.33' - <2'	0.48	0.68	0.60	0.24	0.24	0.24	0.39	0.48		-
┝	54						'	2' - <3'	0.48	0.68	0.60	0.12	-	-	-	-	-	54
ŀ	49 49		9' x 6'	10	10	10	to	3' - <5' 5' - 10'	0.45	0.57	0.56 0.56	0.12	-		_	-		49
ŀ	49		9 x 6					3 - 10 15'	0.52	0.53	0.81	0.12	-		_	-		49
┝	44						12	20'	0.74	1.07	1.07	0.12	_		_			44
╁	44			10	10.5	10	8 to	25'	1.18	1.35	1.28	0.12	_		_		-	44
╁	44			11	11.5	10	12	30'	1.10	1.44	1.38	0.12	_		_		-	44
ŀ	-	\vdash		1 1	11.5	10	12	0.33' - <2'	0.49	0.70	0.63	0.12	0.24	0.24	0.39	0.49	. 2	
╁	59						4	2' - <3'	0.49	0.70	0.63	0.24	-	0.24	-	-	I	59
ŀ	54							3' - <5'	0.49	0.59	0.59	0.12	_	_			Note	54
╁	49		9' x 7'	10	10	10	to	5' - 10'	0.54	0.57	0.60	0.12	_		_	General	49	
ŀ	44		<i>3 </i>					15'	0.75	0.84	0.86	0.12	_	_	_		44	
ŀ	44						12	20'	0.98	1.13	1.14	0.12	_	_	_			44
╁	44			10	10.5	10	8 to	25'	1.18	1.43	1.36	0.12	_	_	_	_		44
ŀ	44			11	11.5	10	12	30'	1.28	1.52	1.46	0.12	_	_	_	_	See	44
r	_							0.33' - <2'	0.51	0.72	0.65	0.24	0.24	0.24	0.39	0.51	1	
t	59						4	2' - <3'	0.51	0.72	0.65	0.12	-	-	-	-	1	59
ı	59							3' - <5'	0.49	0.61	0.62	0.12	-	_	_	_		59
T	54		9' x 8'	10	10	10	to	5' - 10'	0.57	0.60	0.65	0.12	-	-	-	_		54
ı	44							15'	0.79	0.89	0.92	0.12	-	-	-	-	1	44
T	44						12	20'	1.02	1.20	1.22	0.12	-	-	-	ı	1	44
T	44			10	10.5	10	8 to	25'	1.21	1.50	1.44	0.12	-	_	-	-		44
Γ	44			11	11.5	10	12	30'	1.33	1.59	1.54	0.12	-	-	-	-		44
Γ	-						_	0.33' - <2'	0.54	0.74	0.68	0.24	0.24	0.24	0.41	0.54		-
Γ	72						4	2' - <3'	0.54	0.74	0.68	0.15	-	_	-	-		72
Γ	72							3' - <5'	0.53	0.63	0.64	0.13	-	-	-	-	1	72
	59		9' x 9'	10	10	10	to	5' - 10'	0.62	0.64	0.70	0.12	-	_	-	ı]	59
	49						1.5	15'	0.85	0.94	0.99	0.12	-	-	-	ı		49
	49			<u></u>			12	20'	1.09	1.26	1.29	0.12	-	-	-	ı		49
	44			10	10.5	10	8 to	25'	1.28	1.56	1.52	0.12	-	-	-	-		44
	44	L		11	11.5	10	12	30'	1.42	1.66	1.66	0.12	-	-	-	-		44

NOTES:

- 1. See Sheet 2 for General Notes.
- 2. See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

	ABLE	14 - 5	STANI	DARD P	RECAST BO	х си	LVERT	T DES	SIGNS	(3" (COVE	R) - 10)' SPA	NS	
SPAN x RISE (S) (R)	SLAE TOP	вот.	1	HAUNCH	DESIGN EARTH COVER ABOVE			R	REINFOF (s	RCEMEN q. in./F		15			As1 EXT LENGTH (M)
(Ft.)	(Tt) (in.)	(Tb) (in.)	(Tw) (in.)	(H) (in.)	TOP SLAB	As1	As2	As3	As4	As5	As6	As7	As8	A59	(in.)
					0.33' - <2'	0.60	0.73	0.61	0.24	0.24	0.24	0.50	0.57	7133	_
				4	2' - <3'	0.60	0.73	0.61	0.12	-	-	-	-		58
				to	3' - <5'	0.57	0.64	0.58	0.12	_	_	_	_		53
10' x 5'	10	10	10	12	5' - 10'	0.65	0.60	0.60	0.12	_	_	_	_		52
10 10	'	10	10	1 12	15'	0.94	0.90	0.89	0.12	_	_	_	_		47
	10	10	10	8	20'	1.24	1.23	1.19	0.12	_	_	_	_		47
	11	11.5	10	to	25'	1.39	1.37	1.28	0.12	_	_	_	_		47
	12.5	12.5	10	12	30'	1.38	1.43	1.41	0.12	_	_	_	_		47
	12.5	12.5	10	1 12	0.33' - <2'	0.58	0.75	0.64	0.12	0.24	0.24	0.48	0.56		-
				4	2' - <3'	0.58	0.75	0.64	0.12	-	-	-	-		58
				to	3' - <5'	0.56	0.73	0.62	0.12		_		_		52
10' x 6'	10	10	10	12	5' - 10'	0.64	0.64	0.65	0.12	_	_		_		52
10 x 0	10	10	10	12	15'	0.92	0.96	0.03	0.12	_	_	_	_		47
	10	10	10	8	20'	1.21	1.31	1.27	0.12	_	_	_	_		47
	11	11.5	10	to	25'	1.35	1.44	1.36	0.12	_	_	_	_		47
	12.5	12.5	10	12	30'	1.35	1.44	1.49	0.12	_	_	_	_		47
	12.5	12.5	10	12				l							47
				1	0.33' - <2'	0.57	0.78	0.67	0.24	0.24	0.25	0.48	0.57		-
				4	2' - <3'	0.57	0.78	0.67	0.12	-	-	-	-		58
101 71	1.0	1.0	1 , ,	to	3' - <5'	0.58	0.70	0.65	0.12	_	-	-	-		58
10' x 7'	10	10	10	12	5' - 10'	0.65	0.68	0.70	0.12	-	-	-	-		52
	10	10	10	8	15'	0.92	1.02	1.02	0.12	-	-	-	-	5 5	47
	10	10	10	4	20'	1.21	1.38	1.35	0.12	-	-	-	-	Note	47
	11	11.5	10	to	25'	1.33	1.52	1.44	0.12	-	-	-	-	2	47
	12.5	12.5	10	12	30'	1.38	1.58	1.57	0.12	-	-	-	-	General	47
					0.33' - <2'	0.58	0.80	0.70	0.24	0.26	0.24	0.48	0.58	ene	-
				4	2' - <3'	0.58	0.80	0.70	0.12	-	-	-	-		64
				to	3' - <5'	0.60	0.72	0.68	0.12	-	-	-	-	See	58
10' x 8'	10	10	10	12	5' - 10'	0.67	0.72	0.75	0.12	-	-	-	-	,	52
					15'	0.95	1.08	1.08	0.12	-	-	-	-		47
	10	10	10	8	20'	1.24	1.45	1.44	0.12	-	-	-	-		47
	11	11.5	10	to	25'	1.36	1.59	1.52	0.12	-	-	-	-		47
	12.5	12.5	10	12	30'	1.45	1.64	1.64	0.12	-	-	-	-		47
					0.33' - <2'	0.61	0.82	0.73	0.24	0.26	0.24	0.50	0.61		_
				4	2' - <3'	0.61	0.82	0.73	0.14	-	-	-	-		70
				to	3' - <5'	0.64	0.75	0.73	0.13	-	-	-	-		64
10' x 9'	10	10	10	12	5' - 10'	0.72	0.77	0.80	0.12	-	-	-	-		58
					15'	1.00	1.13	1.15	0.12	-	-	-	-		52
	10	10	10	8	20'	1.30	1.53	1.52	0.12	-	-	-	-		47
	11	11.5	10	to	25'	1.42	1.66	1.60	0.12	-	-	-	-		47
	12.5	12.5	10	12	30'	1.57	1.70	1.72	0.12	-	-	-	-		47
					0.33' - <2'	0.66	0.84	0.75	0.24	0.27	0.24	0.52	0.65		_
				4	2' - <3'	0.66	0.84	0.75	0.20	-	-	-	-		79
				to	3' - <5'	0.70	0.77	0.79	0.19	-	-	-	-		70
10' × 10'	10	10	10	12	5' - 10'	0.79	0.81	0.87	0.18	-	-	-	-		64
					15'	1.09	1.19	1.23	0.15	-	-	-	-		52
	10	10	10	8	20'	1.40	1.61	1.61	0.14	-	-	-	-		52
	11	11.5	10	to	25'	1.53	1.74	1.68	0.14	-	-	_	-		47
	12.5	12.5	10.5	12	30'	1.60	1.71	1.74	0.14	-	-	-	-		47
														-	

T.	ABLE	<i>15 -</i> 3	STANI	DARD F	PRECAST BO	X CU	LVER	T DES	SIGNS	(3" (COVE	R) - 11	!' SPA	NS	
SPAN x RISE	SLAB	/ WAL	L THIC	KNESS	DESIGN			R	EINFOR	RCEMEN	T AREA	15			As1 EXT.
(S) (R)	TOP	вот.	SIDE	HAUNCH	EARTH COVER				(5	q. in./F	t.)				LENGTH
(51.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE										(M)
(Ft.)	(in.)	(in.)	(in.)	(in.)	TOP SLAB	As 1	As2	A53	As4	As5	A56	As7	As8	A59	(in.)
				4	0.33' - <2'	0.60	0.66	0.54	0.27	0.27	0.27	0.52	0.56		-
				,	2' - <3'	0.60	0.66	0.54	0.14	-	-	-	-		62
				to	3' - <5'	0.60	0.61	0.53	0.14	-	-	-	-		62
11' × 4'	11	11	11		5' - 10'	0.79	0.63	0.62	0.14	-	-	-	-		55
				12	15'	1.01	0.82	0.79	0.14	-	-	-	-		55
					20'	1.34	1.11	1.06	0.14	-	-	-	-		55
	12	12	11	8 to	25'	1.52	1.27	1.23	0.14	-	-	-	-		55
	13.5	13.5	11	12	30'	1.54	1.37	1.34	0.14	-	-	-	-		50
					0.33' - <2'	0.57	0.71	0.60	0.27	0.27	0.27	0.47	0.53		-
				4	2' - <3'	0.56	0.71	0.60	0.14	-	-	-	-		62
				to	3' - <5'	0.56	0.67	0.59	0.14	-	-	-	-		55
11' x 6'	11	11	11	12	5' - 10'	0.73	0.71	0.72	0.14	-	-	-	-		55
				_	15'	0.92	0.92	0.91	0.14	-	-	-	-		50
	11	11	11	8	20'	1.21	1.25	1.21	0.14	-	-	-	-		50
	12	12	11	to	25'	1.37	1.43	1.39	0.14	-	-	-	-		50
	13.5	13.5	11	12	30'	1.39	1.53	1.50	0.14	-	-	-	-		50
					0.33' - <2'	0.55	0.76	0.66	0.27	0.27	0.27	0.46	0.55	5	-
				4	2' - <3'	0.55	0.76	0.66	0.14	-	-	-	-	Note	62
				to	3' - <5'	0.54	0.72	0.65	0.14	-	-	-	-		62
11' × 8'	11	11	11	12	5' - 10'	0.73	0.79	0.82	0.14	-	-	-	-	General	55
					15'	0.93	1.03	1.03	0.14	-	-	-	-	ine	50
	11	11	11	8	20'	1.21	1.39	1.36	0.14	-	-	-	-		50
	12	12.5	11	to	25'	1.34	1.56	1.50	0.14	-	-	-	-	See	50
	13.5	13.5	11	12	30'	1.41	1.66	1.65	0.14	-	-	-	-	2	50
					0.33' - <2'	0.60	0.81	0.71	0.27	0.27	0.27	0.48	0.60		-
				4	2' - <3'	0.60	0.81	0.71	0.15	-	-	-	-		75
			l	to	3' - <5'	0.61	0.77	0.70	0.14	-	-	-	-		69
11' × 10'	11	11	11	12	5' - 10'	0.80	0.88	0.93	0.14	-	-	-	-		62
				0	15'	1.01	1.13	1.15	0.14	-	-	-	-		55
	11	11	11	8	20'	1.30	1.52	1.52	0.14	-	-	-	-		50
	12	12.5	11	to	25'	1.42	1.70	1.65	0.14	-	_	-	-		50
	13.5	14	11	12	30'	1.53	1.77	1.74	0.14	-	-	-	-		50
					0.33' - <2'	0.64	0.83	0.74	0.27	0.27	0.27	0.51	0.64		-
				4	2' - <3'	0.64	0.83	0.74	0.21	-	=	-	=		86
			١	to	3' - <5'	0.67	0.79	0.75	0.21	-	-	-	-		75
11' × 11'	11	11	11	12	5' - 10'	0.88	0.93	0.99	0.19	-	-	-	-		69
					15'	1.09	1.19	1.23	0.16	-	-	-	-		55
	11	11	11	8	20'	1.40	1.59	1.60	0.15	-	-	-	-		55
	12	12.5	11	to	25'	1.54	1.77	1.73	0.15	-	-	-	-		50
	13.5	14	11.5	12	30'	1.57	1.77	1.76	0.14	-	-	-	-		50

NOTES:

- See Sheet 2 for General Notes.
 See Sheet 7 for Reinforcing Details and dimension locations.
- 3. See Sheet 14 for WWR Bending Diagrams.

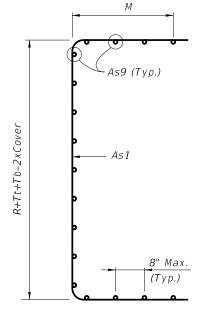
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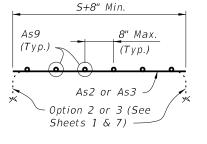


T.	ABLE	16 - 3	STANI	DARD P	RECAST BO	X CU	LVERT	DES	SIGNS	(3" C	COVER	R) - 12	2' SPA	ANS	
SPAN x RISE	SLAE		LTHIC		DESIGN			R		RCEMEN		A <i>S</i>			As1 EXT.
(S) (R)	TOP	BOT.			EARTH COVER				(5	q. in./F	t.)				LENGTH
(Ft.)	(Tt)	(Tb)	(Tw)	(H)	ABOVE TOP SLAB						Г		Т		(M) (in.)
(1 (.)	(in.)	(in.)	(in.)	(in.)		As 1	As2	As3	As4	As5	A56	As7	A58	A59	<u> </u>
				4	0.33' - <2'	0.59	0.64	0.51	0.29	0.29	0.29	0.52	0.55		-
				to	2' - <3'	0.60	0.64	0.51	0.15	-	-	-	-		73
	12	12	12		3' - <5'	0.60	0.61	0.51	0.15	_	-	-	-		66
12' x 4'					5' - 10'	0.81	0.61	0.61	0.15	-	-	-	-	-	66
				12	15' 20'	1.04	0.80	0.77	0.15	-	-	-	-	-	59
	12	1 7	12	0 to	20° 25'	1.37	1.08	1.03	0.15	-	-	-	-		59
	13 14.5	13 14.5	12	8 to 12	30'	1.58 1.63	1.26 1.38	1.21 1.34	0.15	-	-	 -	-		59 53
	14.5	14.5	12	12	0.33' - <2'	0.56	0.70	0.57	0.13	- 0.29	- 0.29	0.47	- 0.52	{	-
				4	2' - <3'	0.56	0.70	0.57	029	0.29	0.29	-	-		66
					3' - <5'	0.56	0.70	0.57	0.15	_	_	 -	_	-	59
	12	12	12	to	5' - 10'	0.74	0.69	0.70	0.15	_	_	- -	_		59
12' x 6'					15'	0.74	0.90	0.70	0.15	_	_	-	_	-	53
				12	20'	1.23	1.22	1.17	0.15	_	_	<u> </u>	_	-	53
	13	13	12	8 to	25'	1.40	1.42	1.37	0.15	_	_	_	_	-	53
	14.5	15	12	12	30'	1.44	1.54	1.48	0.15	_	_		_	-	53
					0.33' - <2'	0.55	0.75	0.63	0.29	0.29	0.29	0.45	0.53	- 2	
				4	2' - <3'	0.55	0.75	0.63	0.15	-	-	-	- S	1	66
	1.2	1.0	1.5	to	3' - <5'	0.55	0.73	0.63	0.15	_	_	_		No	59
1.2/ 0/	12	12	12	12	5' - 10'	0.73	0.77	0.79	0.15	_	_	-	-		59
12' x 8'					15'	0.93	1.00	0.99	0.15	-	-	-	-	General	53
	12	12	12	8	20'	1.21	1.35	1.31	0.15	_	-	-	-	<i>Ge</i> /	53
	13	13.5	12	to	25'	1.35	1.55	1.48	0.15	-	-	-	-	See	53
	14.5	15	12	12	30'	1.40	1.67	1.62	0.15	-	-	-	-	Se	53
					0.33' - <2'	0.57	0.80	0.68	0.29	0.29	0.29	0.46	0.57		-
				4	2' - <3'	0.57	0.80	0.68	0.15	-	-	-	-		73
	12	12	12	to	3' - <5'	0.59	0.77	0.68	0.15	-	-	-	-		66
12' x 10'	12	12	12	12	5' - 10'	0.78	0.85	0.89	0.15	-	-	-	-		59
12 % 10					15'	0.98	1.10	1.11	0.15	-	-	-	-		53
	12	12	12	8	20'	1.26	1.47	1.45	0.15	-	-	-	-		53
	13	13.5	12	to	25'	1.39	1.68	1.63	0.15	-	-	-	-		53
	14.5	15	12	12	30'	1.48	1.79	1.76	0.15	-	-	-	-		53
					0.33' - <2'	0.65	0.84	0.73	0.29	0.29	0.29	0.50	0.65		-
				4	2' - <3'	0.65	0.84	0.73	0.23	-	-	-	-		93
	12	12	12	to	3' - <5'	0.68	0.81	0.75	0.22	-	_	-	_		80
12' x 12'				12	5' - 10'	0.90	0.94	1.01	0.21	-	_	-	-		73
	<u> </u>	1.0	1.0		15'	1.12	1.20	1.24	0.18	=	-	-	-		59
	12	12	12	8	20'	1.42	1.60	1.61	0.16	=	=	-	=		59
	13	13.5	12	to	25'	1.57	1.81	1.78	0.16	_	-	-	-		53
	14.5	15	12.5	12	30'	1.63	1.86	1.85	0.15	-	-	_	_		53

- 1. See Sheet 2 of 14 for General Notes.
- 2. See Sheet 7 of 14 for Reinforcing Details and dimension locations.

WELDED WIRE REINFORCEMENT BENDING DIAGRAM



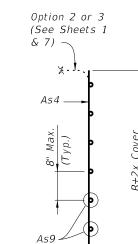


WWR PIECE NO. 2 (2 Reqd. per segment)

S+2(Tw+10"-Cover-M)

8" Max.

(Typ.)



WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (Tongue Reinforcement) (4 Reqd. per segment)

As4 (3 Wires Min.)

WWR PIECE NO. 3 (2 Regd. per segment)

Option 2 or 3

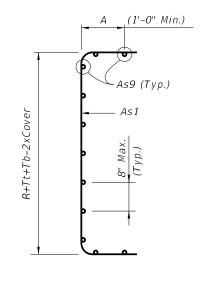
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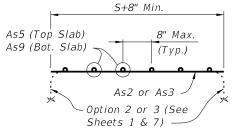
& 7)

TYPE 2 BOX SECTION (DESIGN EARTH COVER 2' OR GREATER)

As9

(Typ.)





WWR PIECE NO. 2 (2 Read. per segment) S+2(Tw+10"-Cover-A) 8" Max. (Typ.)∽As7 (Top Slab) As6 (Top Slab) As8 (Bot. Slab) As9 (Bot. Slab)

×-As4 _ As9 -

WWR PIECE NO. 1 (2 Reqd. per segment)

WWR PIECE NO. 4 (2 Reqd. per segment)

WWR PIECE NO. 3 (2 Read. per segment)

=== TYPE 1 BOX SECTION (DESIGN EARTH COVER LESS THAN 2') =====

REINFORCEMENT NOTES:

- Reinforcement bending dimensions are out-to-out.
- 2. See General Notes 4, 5 and 6 on Sheet 2.
- 3. See Tables 1 thru 16 for dimensions M, R, S, Tb, Tt and Tw.
- 4. Dimension "A" is determined by the Manufacturer in accordance with the requirements of Detail "B" on Sheets 1 and 7.

DESCRIPTION: LAST REVISION 01/01/07



FDOT DESIGN STANDARDS 2013

INDEX NO.

SHEET





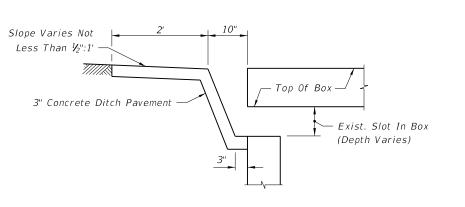




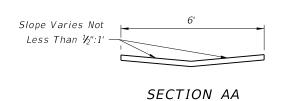


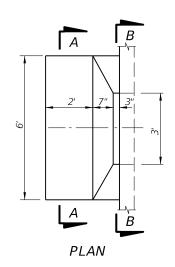


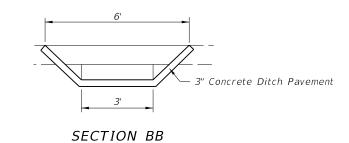
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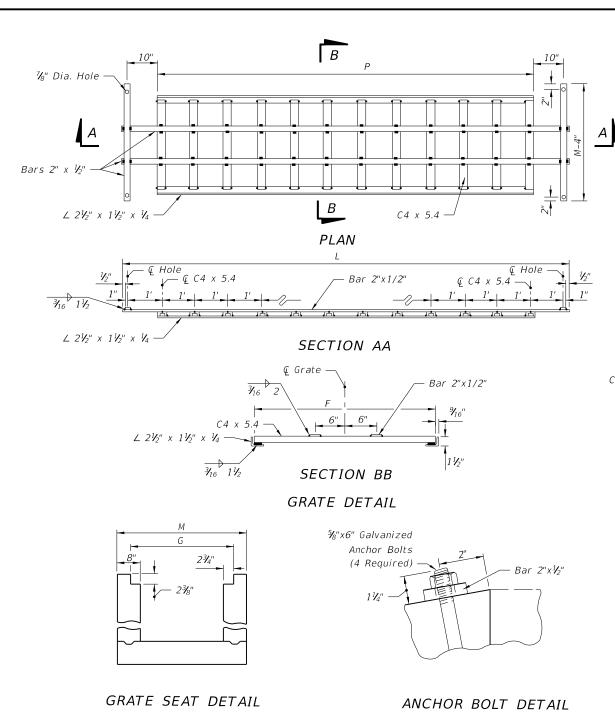
LONGITUDINAL SECTION

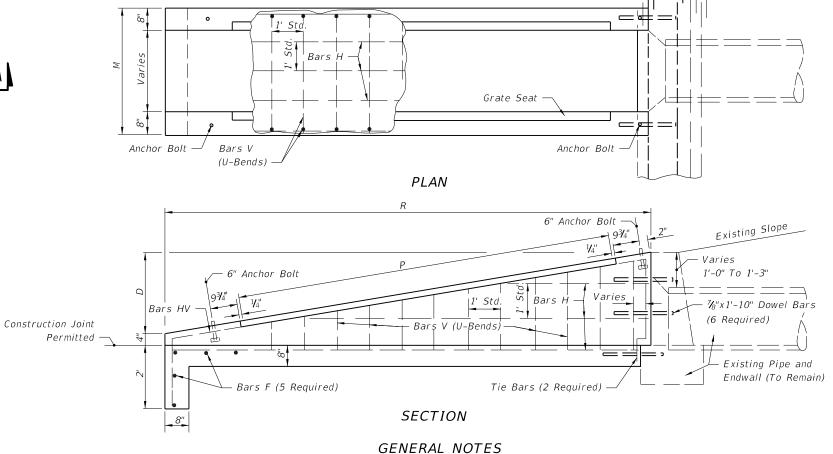






SAFETY MODIFICATION FOR INLETS IN BOX CULVERTS





- 1. For use criteria see "Steel Grating Use Criteria" Index No. 261.
- 2. Grates shall be ASTM A242, A572 or A588, Grade 50 steel, and galvanized in accordance with Section 975 and 425-3.2 of the Standard Specifications.
- 3. Channel section C3 \times 6.0 may be substituted for the C4 \times 5.4 channel.
- 4. All reinforcing No. 4 bars with 2" clearance except as noted. Spacings shown are center to center. Laps to be 12" minimum. Welded wire fabric (two cages max.) having an equivalent cross section area (0.20 sq. in.) may be substituted for bar reinforcement.
- 5. Drill 1¼" holes 8" deep with a rotary drill in existing endwall for dowel bars. Holes shall be thoroughly cleaned prior to installing Adhesive-Bonded Dowels.
- 6. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB. Cost of Adhesive-Bonded Dowels to be included in the contract unit price for reinforcing steel. Cost of grates to be paid for under the contract unit price for Endwall Grate, LB., plan quantity. Cost of galvanized bolts and nuts to be included in the contract unit price for the grate.
- 7. Sod slopes 5' each side and above endwall. Sodding to be paid for under the contract unit price for Performance Turf, SY.

			DIMEN	ISIONS	S AND C	DUANTI	TIES PE	R GRATE		
Slana	Pipe	Channels	@ 5.4 Lb.	s./LF	Bars @	3.4 lbs/L	F (2 ea.)	Angles @	(2)Total	
Slope	Size	Quantity	F	Lbs.	L	M-4"	Lbs.	Р	Lbs.	Weight-Lbs
	15"	10	2'-61/8"	139	11'-3"	3'-3"	99	9'-4"	60	298
1:6	18"	12	2'-91/8"	183	13'-3"	3'-6"	114	11'-4"	73	370
1.0	24"	15	3'-31/8"	269	16'-3"	4'-0"	138	14'-4"	92	499
	30"	18	3'-91/8"	372	19'-3"	4'-6"	162	17'-4"	111	645
	15"	6	2'-67/8"	83	7'-3"	3'-3"	71	5'-4"	34	188
1.1	18"	7	2'-97/8"	107	8'-3"	3'-6"	80	6'-4"	41	228
1:4	24"	9	3'-3 ⁷ / ₈ "	161	10'-3"	4'-0"	97	8'-4"	53	311
	30"	11	3'-97/8"	227	12'-3"	4'-6"	114	10'-4"	66	407

Pipe	G	М	D	R	P	Class I	Reinforcing	Sod
Size		1-1			,	Concrete-CY	Steel-Lbs.	SY
15"	2'-81/2"	3'-7"	2'-2"	13'-0"	9'-4"	2.12	167	23
18"	2'-111/2"	3'-10"	2'-5"	14'-6"	11'-4"	2.53	173	25
24"	3'-51/2"	4'-4"	2'-11"	17'-6"	14'-4"	3.48	238	29
30"	3'-11½"	4'-10"	3'-5"	20'-6"	17'-4"	4.57	315	32
				ı	1	I		
15"	2'-81/2"	3'-7"	2'-2"	8'-8"	5'-4"	1.44	120	19
18"	2'-111/2"	3'-10"	2'-5"	9'-8"	6'-4"	1.72	130	20
24"	3'-51/2"	4'-4"	2'-11"	11'-8"	8'-4"	2.36	167	22
30"	3'-111/5"	4'-10"	3'-5"	13'-8"	10'-4"	3.09	225	25

LAST REVISION 07/01/07

