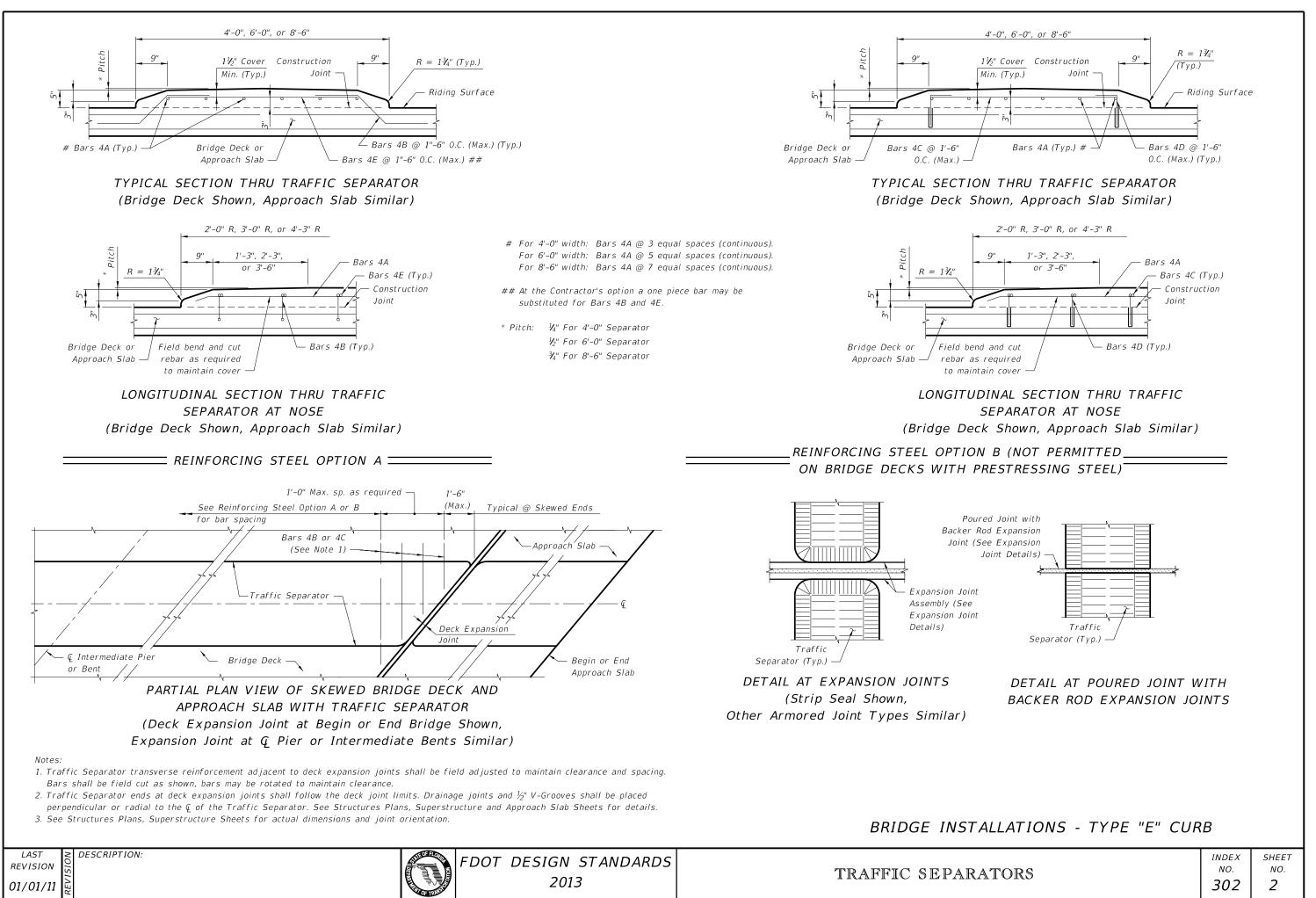
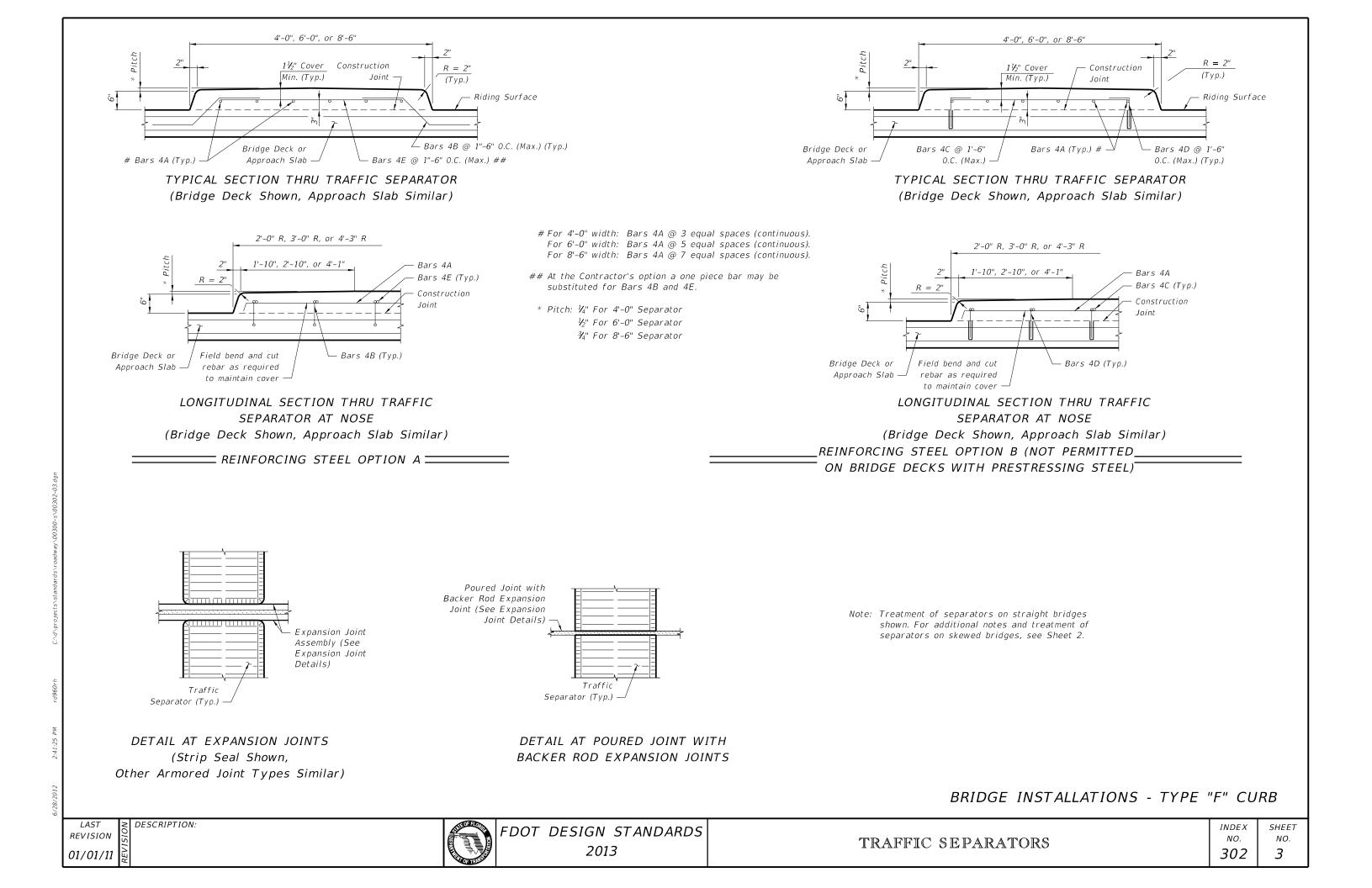
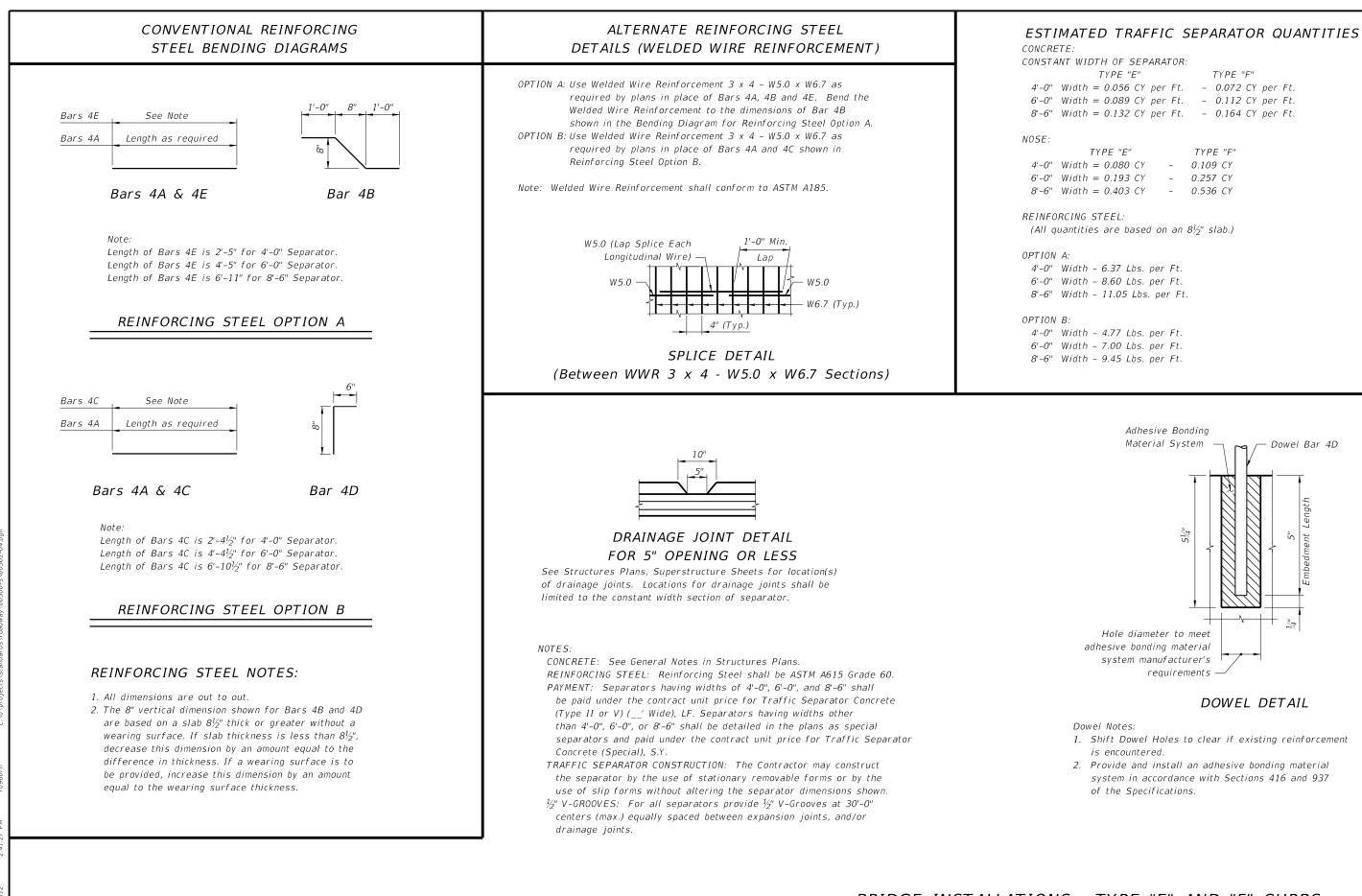


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DESCRIPTION: LAST REVISION

07/01/07

FDOT	DESIGN	STANDARDS 3
	201	3

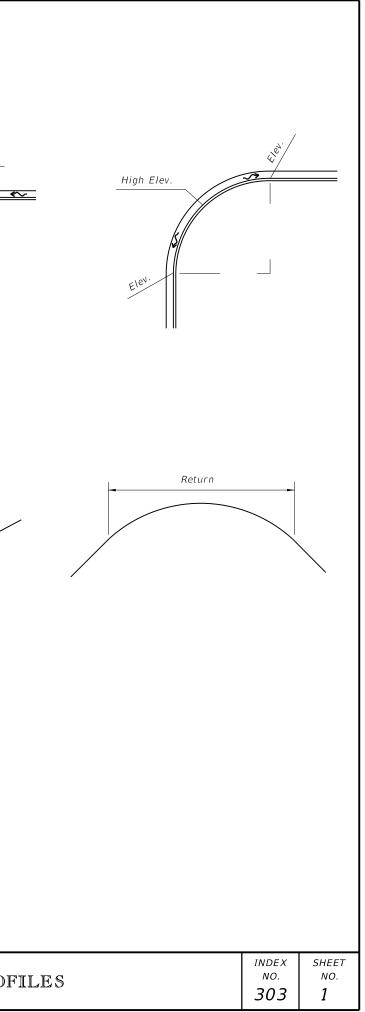
# BRIDGE INSTALLATIONS - TYPE "E" AND "F" CURBS

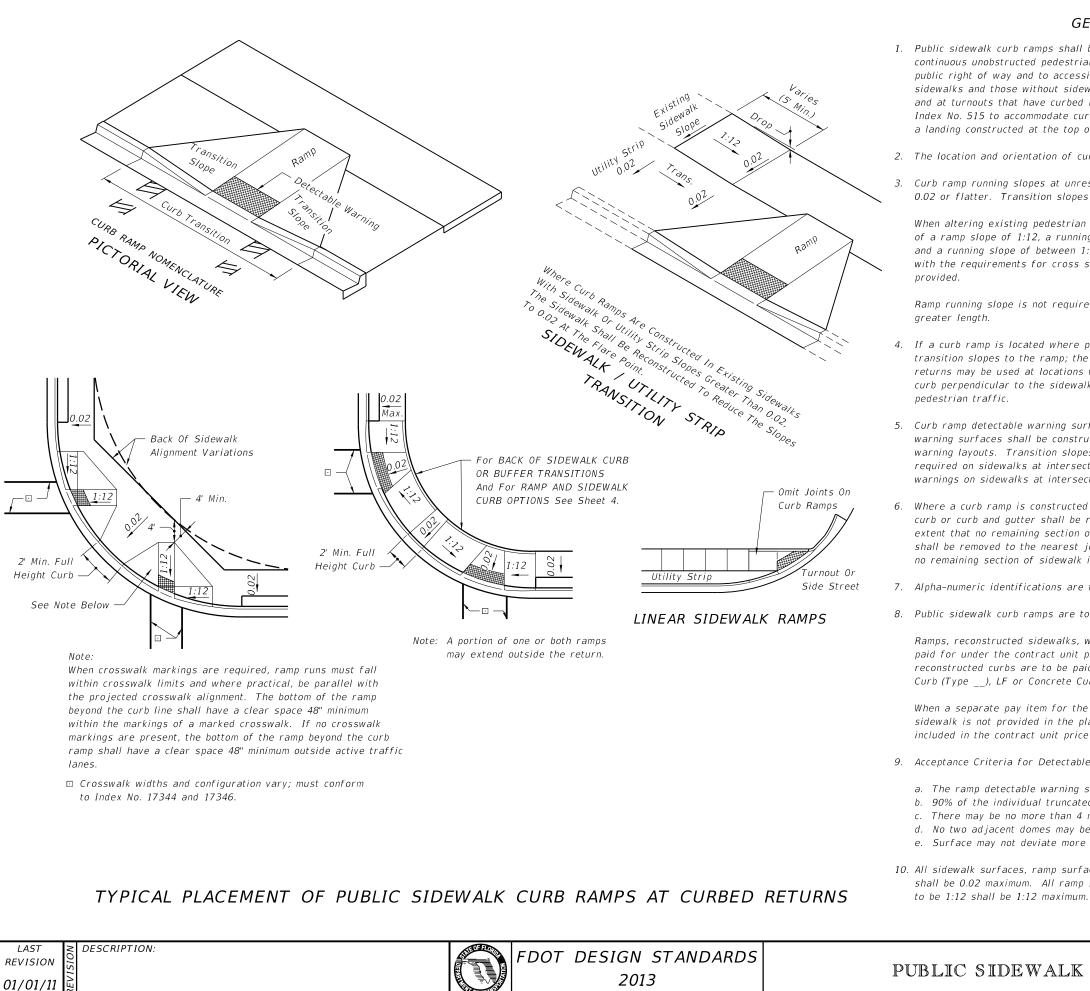
	INDEX	SHEET
TRAFFIC SEPARATORS	NO.	NO.
	302	4

Elev	Elev.	High Elev.
Return	Inlet Return	Return
	PROFILE VIEW	
	Note: Profile grades should be established that will allow inlets to be located outs should be located to avoid conflict with pedestrian movement. Special care m public sidewalk curb ramps for the disabled. For information on public sidew SHOWING LOCATION OF INLETS	ust be exercised to prevent conflict with alk curb ramps refer to Index No. 304.
	TYPICAL RETURN PRO	DFILES
LAST ≥ DESCRIPTION:		
LAST A DESCRIPTION: REVISION 51 01/01/12	FDOT DESIGN STANDARDS 2013	CURB RETURN PRO

2:41:29

6/28/2012





# PUBLIC SIDEWALK CU

## GENERAL NOTES

1. Public sidewalk curb ramps shall be constructed in the public right of way at locations that will provide continuous unobstructed pedestrian circulation paths to pedestrian areas, elements and facilities in the public right of way and to accessible pedestrian routes on adjacent sites. Curbed facilities with sidewalks and those without sidewalks are to have curb ramps constructed at all street intersections and at turnouts that have curbed returns. Partial curb returns shall extend to the limit prescribed by Index No. 515 to accommodate curb ramps. Ramps constructed at locations without sidewalks shall have a landing constructed at the top of each ramp, see Sheet 5.

2. The location and orientation of curb ramps shall be as shown in the plans.

3. Curb ramp running slopes at unrestrained sites shall not be steeper than 1:12 and cross slope shall be 0.02 or flatter. Transition slopes shall not be steeper than 1:12.

When altering existing pedestrian facilities where existing site development precludes the accommodation of a ramp slope of 1:12, a running slope between 1:12 and 1:10 is permitted for a rise of 6" maximum and a running slope of between 1:10 and 1:8 is permitted for a rise of 3" maximum. Where compliance with the requirements for cross slope cannot be fully met, the minimum feasible cross slope shall be

Ramp running slope is not required to exceed 8' in length, except at sites where the plans specify a

4. If a curb ramp is located where pedestrians must walk across the ramp, then the walk shall have transition slopes to the ramp; the maximum slope of the transitions shall be 1:12. Ramps with curb returns may be used at locations where other improvements provide guidance away from that portion of curb perpendicular to the sidewalk; improvements for guidance are not required at curb ramps for linear

5. Curb ramp detectable warning surfaces shall extend the full width of the ramp and 24" deep. Detectable warning surfaces shall be constructed in accordance with Specification 527. See Sheet 6 for detectable warning layouts. Transition slopes are not to have detectable warnings. Detectable warnings are required on sidewalks at intersecting roads, streets and railroads. For requirements for detectable warnings on sidewalks at intersecting driveways, see Index 310.

6. Where a curb ramp is constructed within existing curb, curb and gutter and/or sidewalk, the existing curb or curb and gutter shall be removed to the nearest joint beyond the curb transitions or to the extent that no remaining section of curb or curb and gutter is less than 5' long. The existing sidewalk shall be removed to the nearest joint beyond the transition slope or walk around or to the extent that no remaining section of sidewalk is less than 5' long. For details of Concrete Sidewalk See Index 310.

7. Alpha-numeric identifications are for reference (plans, permits, etc.).

8. Public sidewalk curb ramps are to be paid for as follows:

Ramps, reconstructed sidewalks, walk around sidewalks, sidewalk landings and sidewalk curbs are to be paid for under the contract unit price for Sidewalk Concrete, ( " Thick), SY. Curb transitions and reconstructed curbs are to be paid for under the contract unit price for the parent curb, i.e., Concrete Curb (Type ), LF or Concrete Curb and Gutter (Type ), LF.

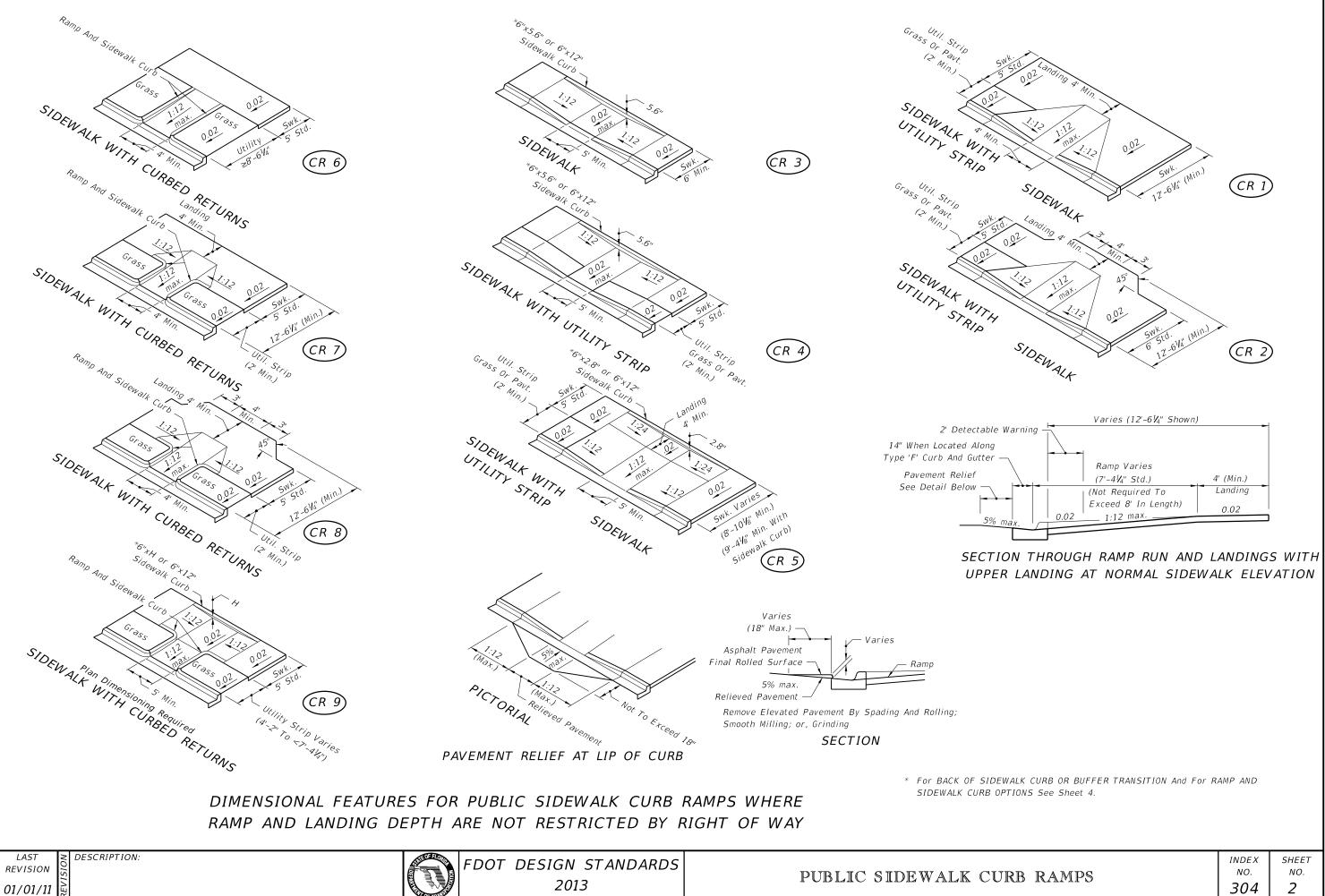
When a separate pay item for the removal and disposal of existing curb, curb and gutter, and/or sidewalk is not provided in the plans, the cost of removal and disposal of these features shall be included in the contract unit price for new curb, curb and gutter and/or sidewalk respectively.

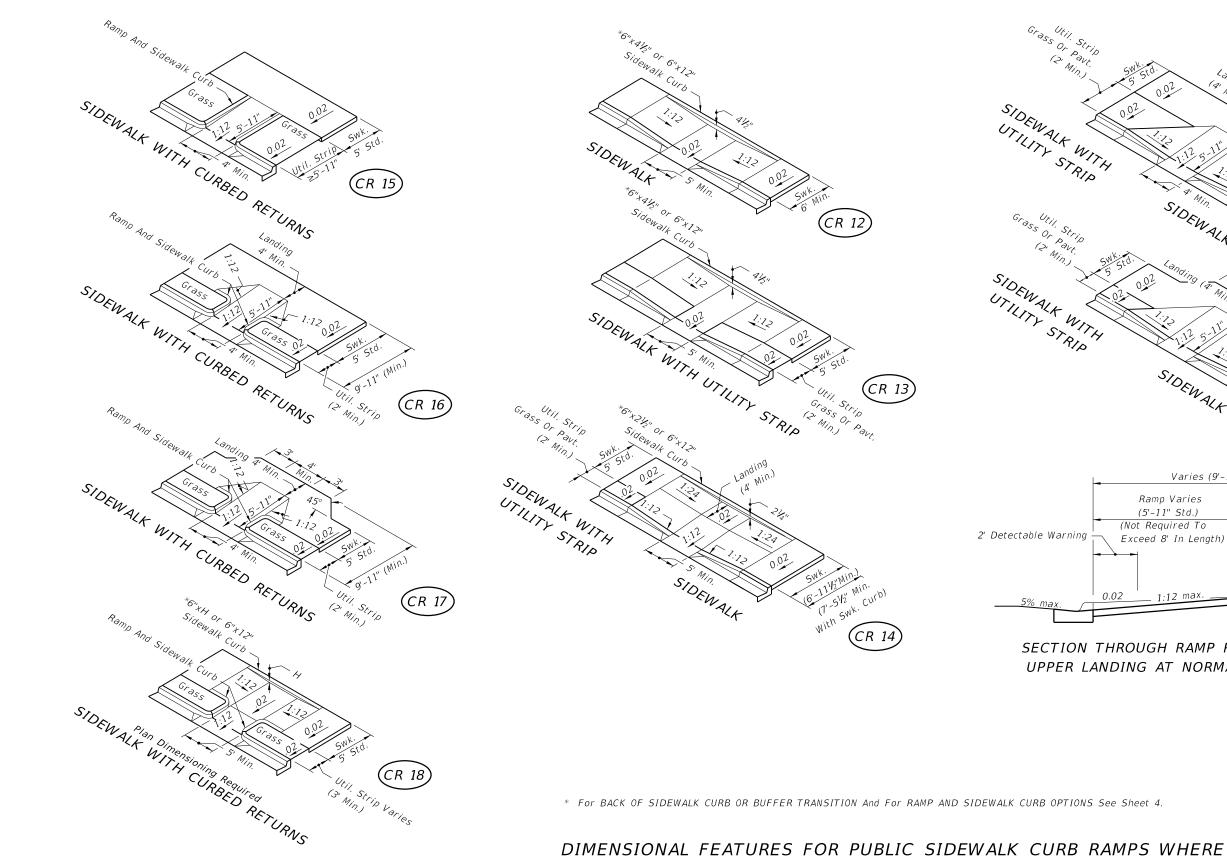
9. Acceptance Criteria for Detectable Warnings:

a. The ramp detectable warning surface shall be complete and uniform in color and texture b. 90% of the individual truncated domes must comply with the design criteria c. There may be no more than 4 non-complying domes in any one square foot of surface d. No two adjacent domes may be non-compliant e. Surface may not deviate more than 0.10" from a true plane

10. All sidewalk surfaces, ramp surfaces, and landings with a cross slope shown in this Index to be 0.02 shall be 0.02 maximum. All ramp surfaces and ramp transition slopes with a slope shown in this Index

		INDEX	SHEET
RB RAMPS	RAMDS	NO.	NO.
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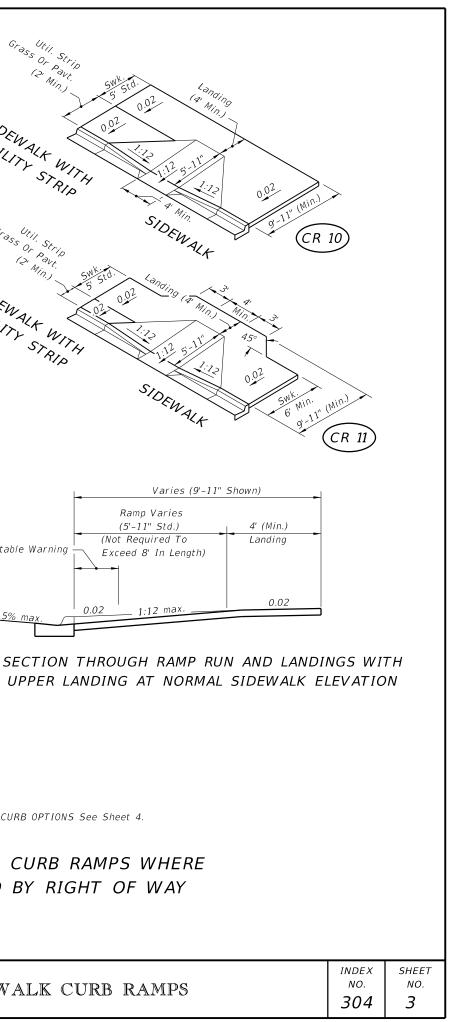


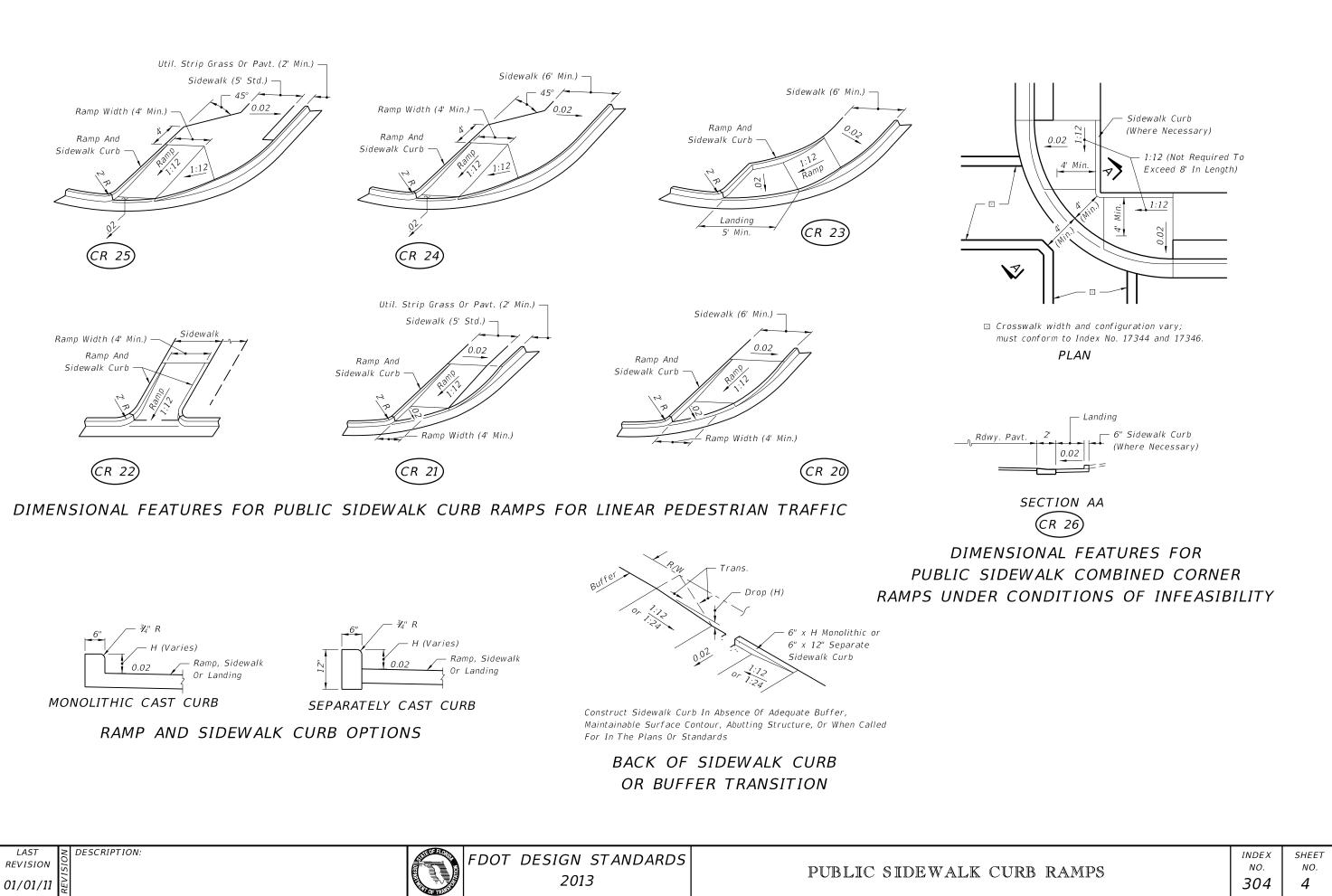
RAMP AND LANDING DEPTH ARE RESTRICTED BY RIGHT OF WAY

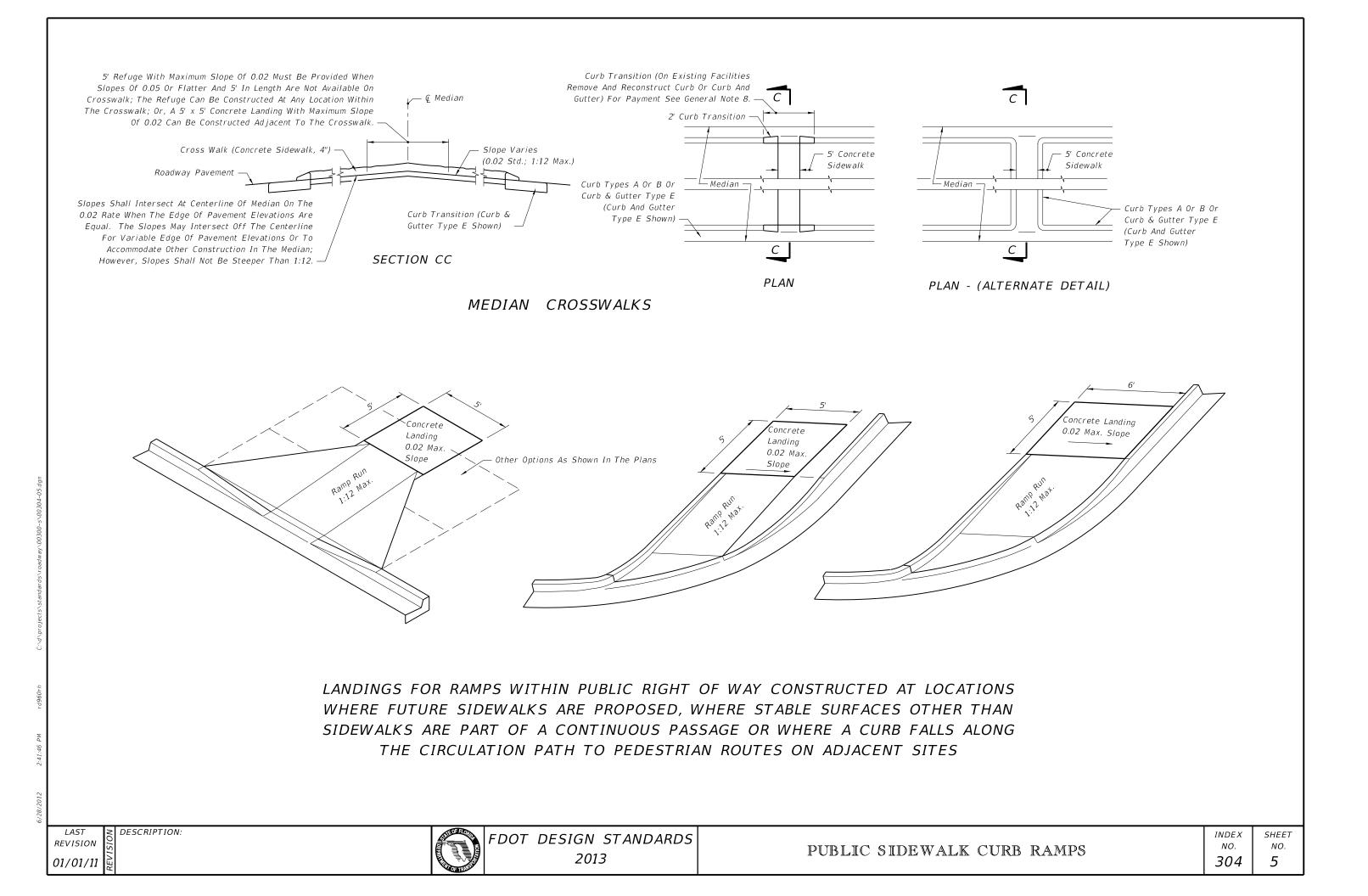
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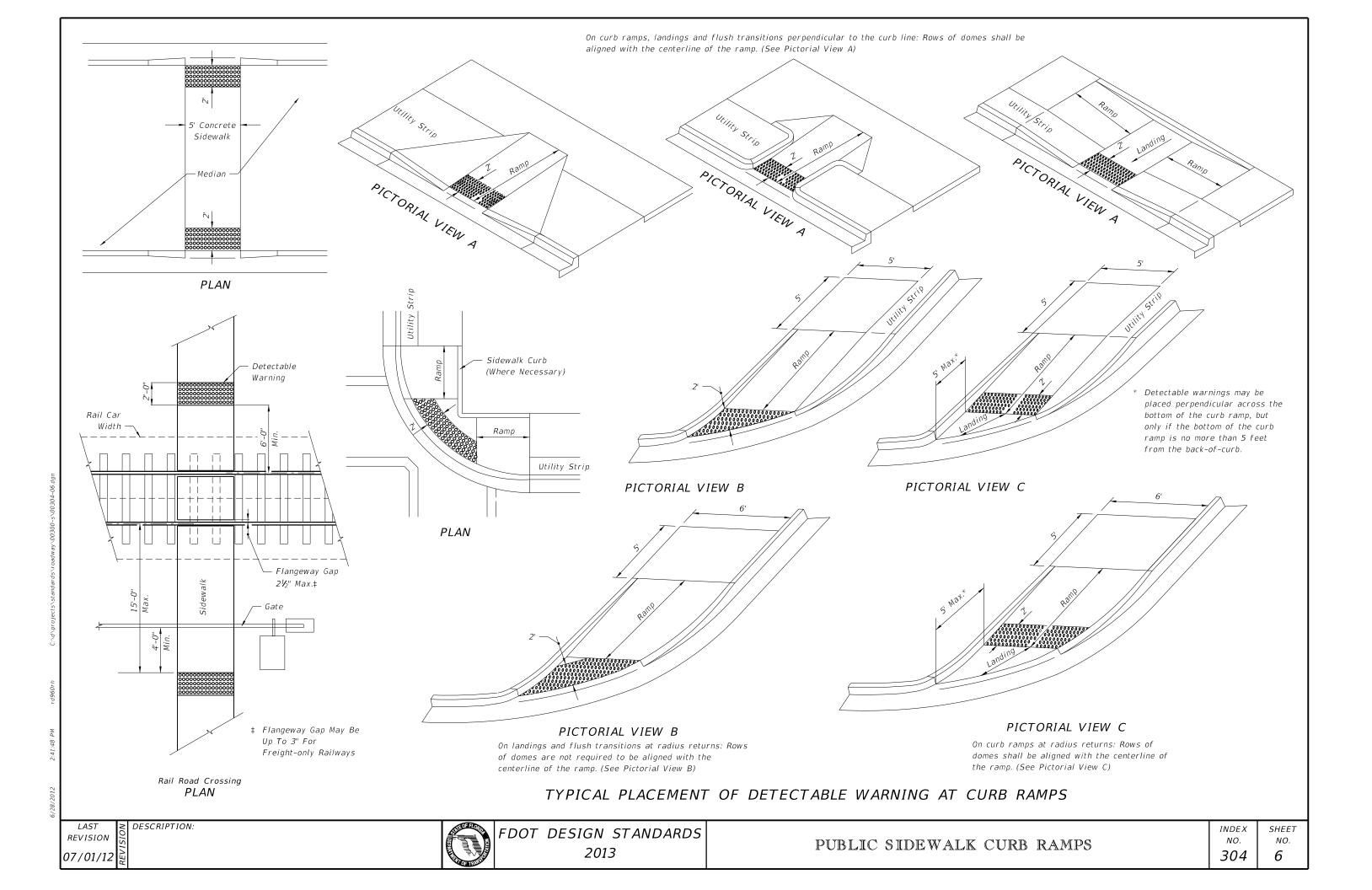
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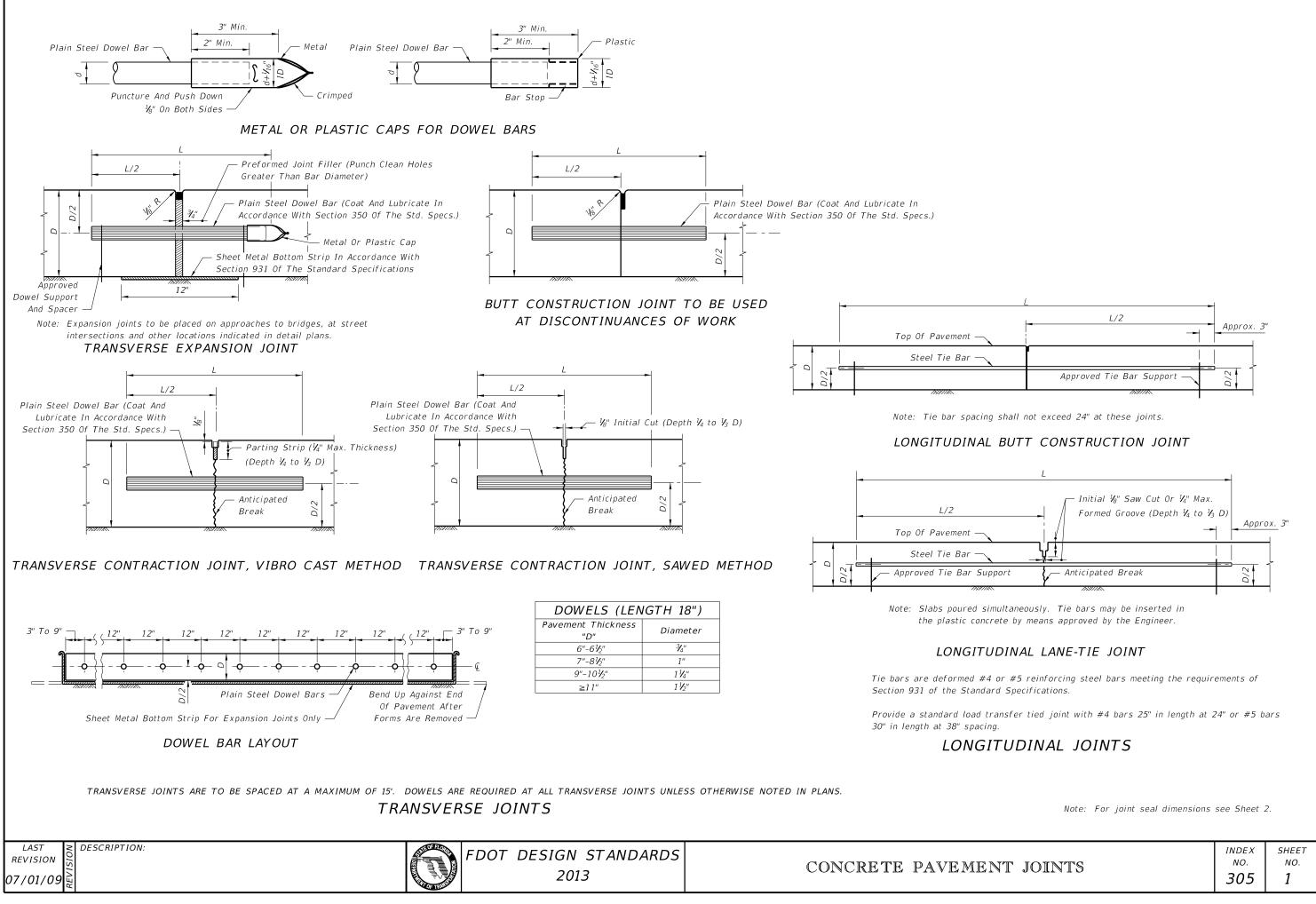
PUBLIC SIDEWALK CURB RAMPS

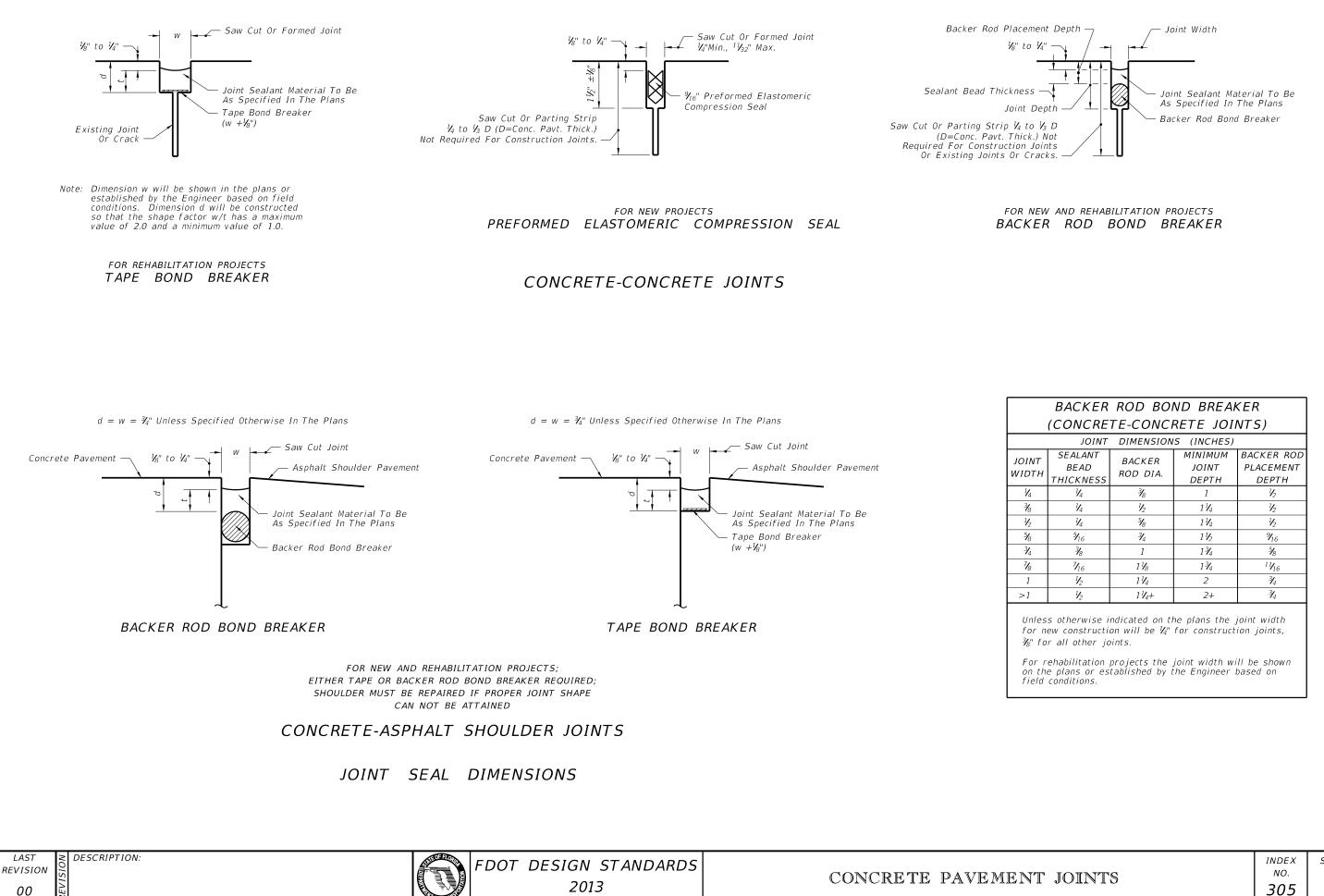






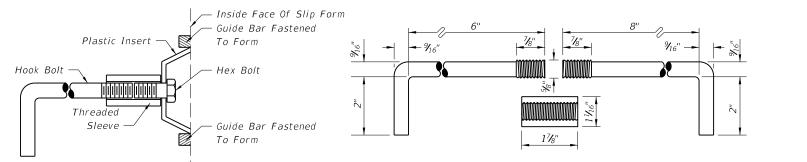




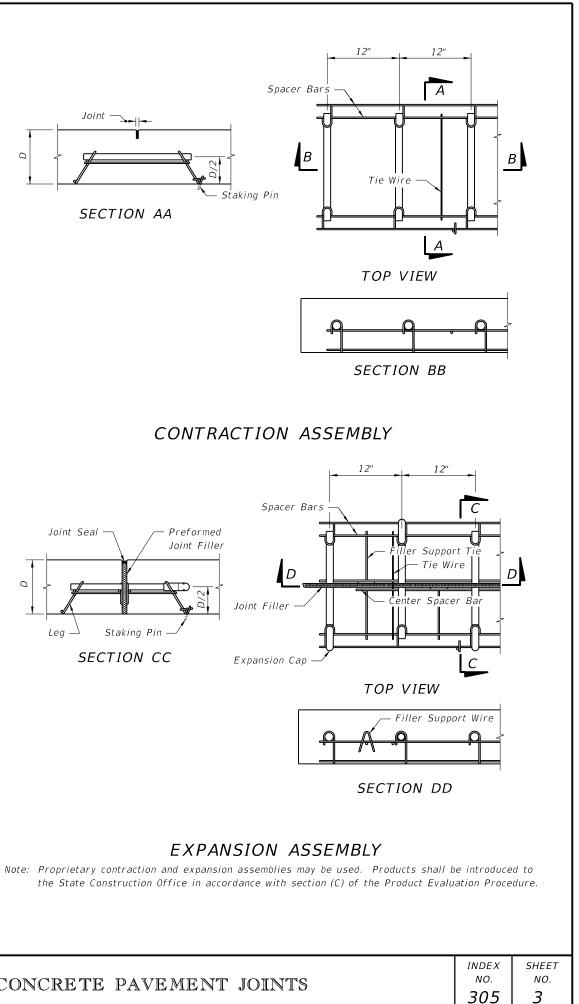


	BACKER ROD BOND BREAKER				
	(CONCRETE-CONCRETE JOINTS)				
	JOINT	DIMENSION	S (INCHES)		
NT TH	SEALANT BEAD THICKNESS	BACKER ROD DIA.	MINIMUM JOINT DEPTH	BACKER ROD PLACEMENT DEPTH	
ı	$V_4$	³∕8	1	$V_2$	
3	$V_4$	$V_2$	1 ¼	₩2	
2	$V_4$	<del>5</del> /8	1 1/4	¥2	
3	<i>5</i> ∕16	¥4	1 ½	91 <sub>16</sub>	
1	∛8	1	1 3⁄4	<del>5</del> /8	
3	7⁄16	1 ½	1 3⁄4	<sup>1</sup> 1/ <sub>16</sub>	
	$V_2$	1 V <sub>4</sub>	2	₹14	
	V <sub>2</sub>	1 V <sub>4</sub> +	2+	₹4	

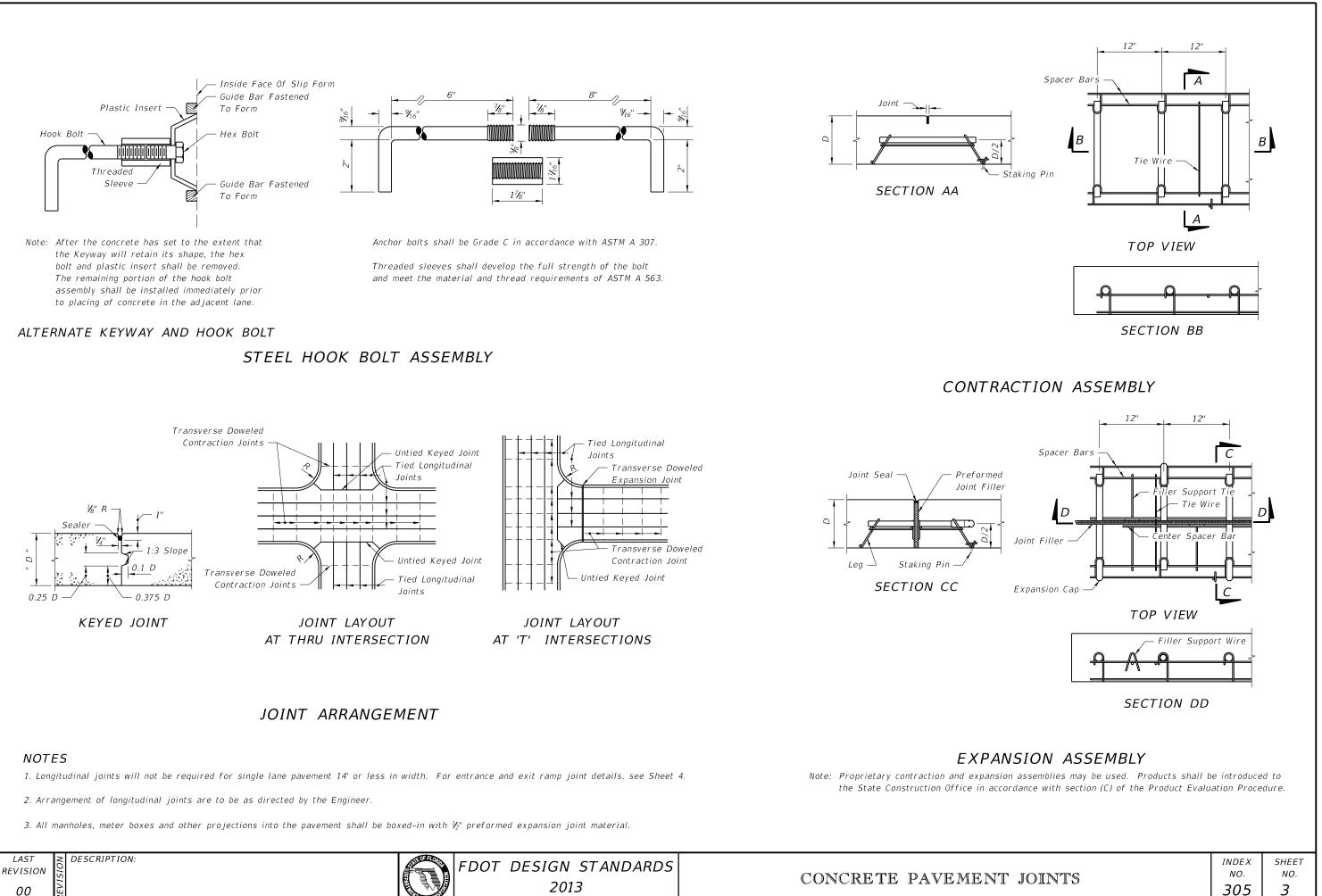
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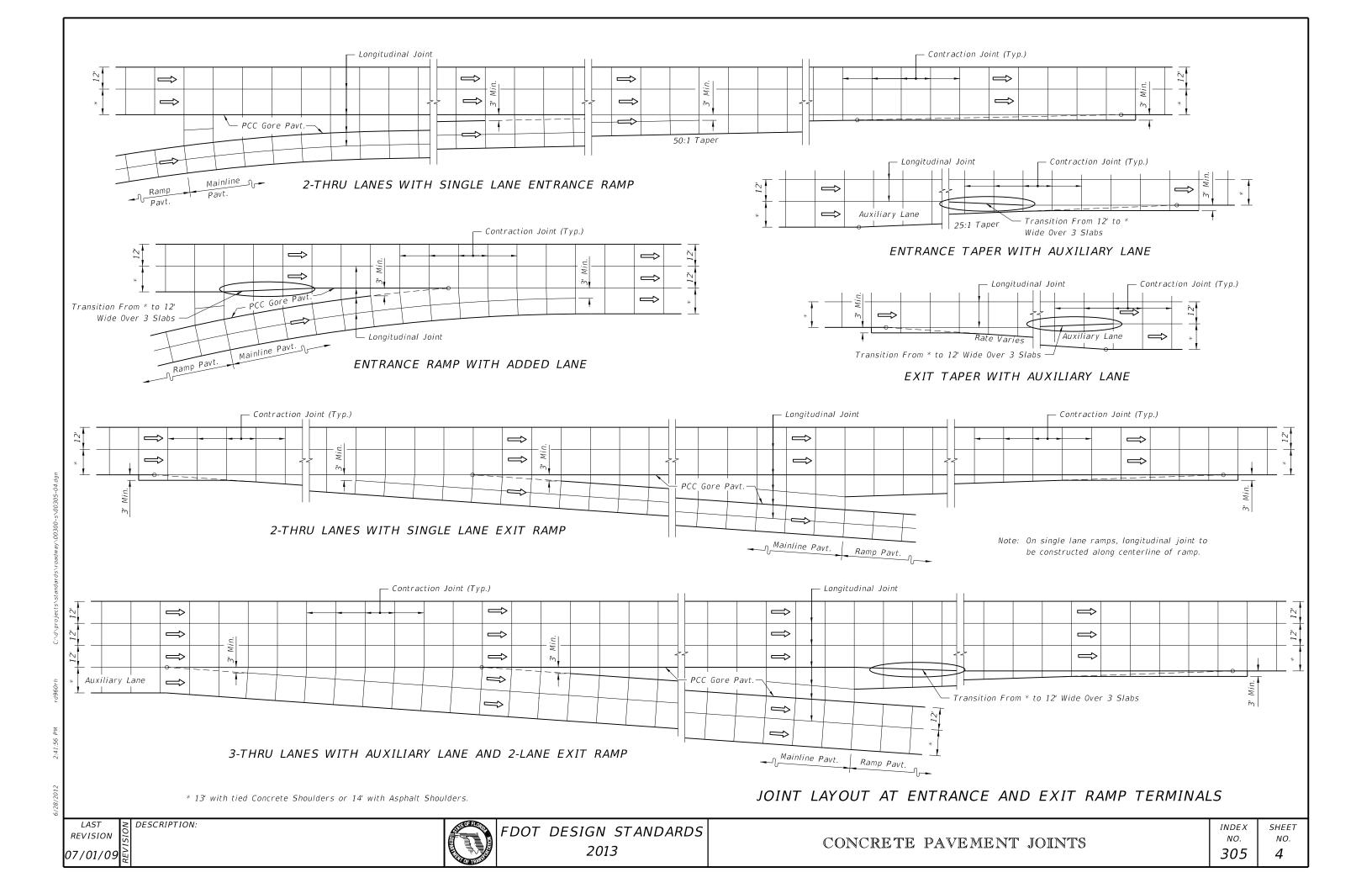


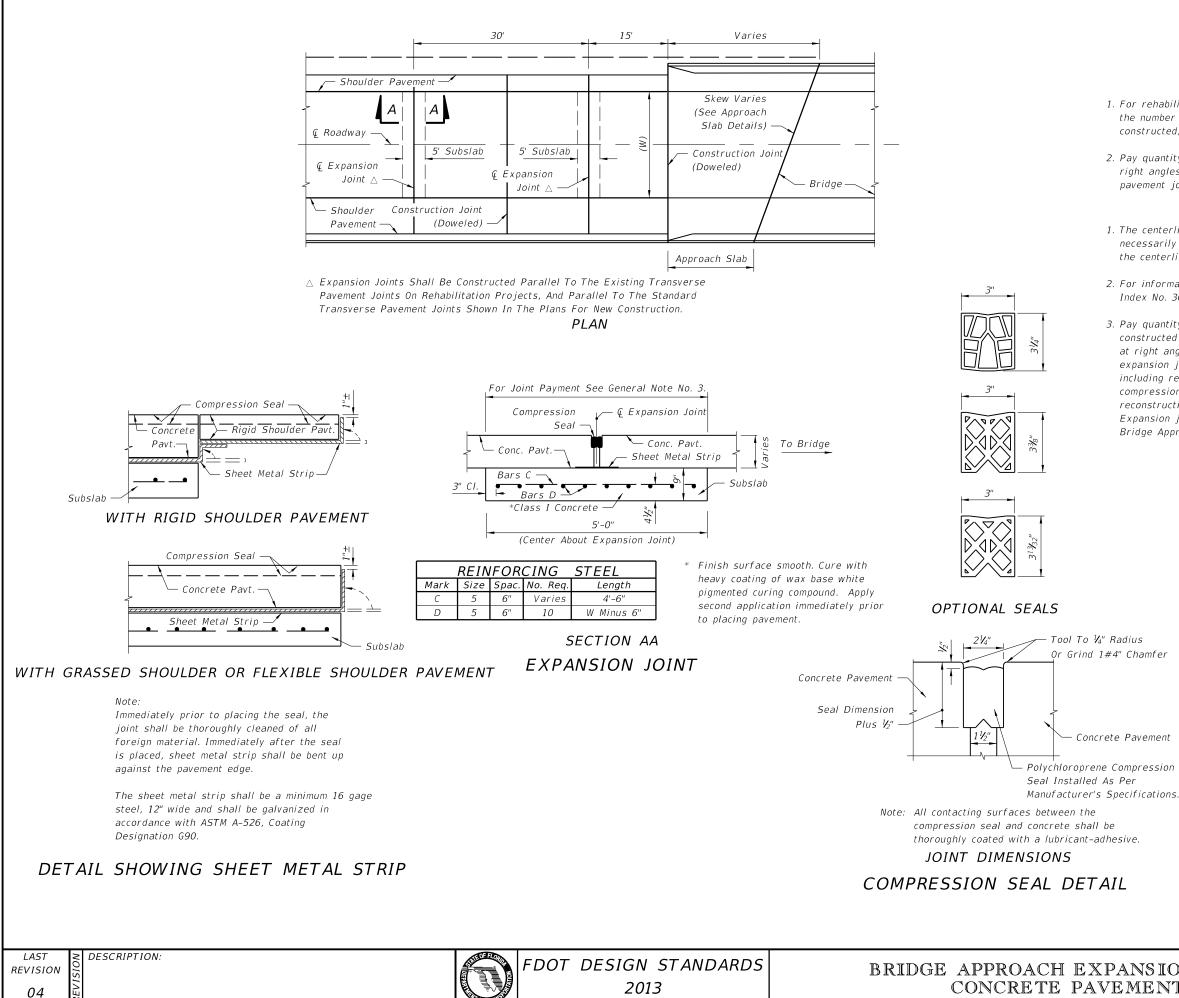
the Keyway will retain its shape, the hex bolt and plastic insert shall be removed. The remaining portion of the hook bolt assembly shall be installed immediately prior to placing of concrete in the adjacent lane.











### DESIGN NOTES

1. For rehabilitation projects, the designer must indicate in the plans the number of slabs to be removed, the number of subslabs to be constructed/reconstructed, and the location of expansion joints.

2. Pay quantity of expansion joint to be calculated across pavement at right angles to the centerline of the roadway pavement. Shoulder pavement joint included.

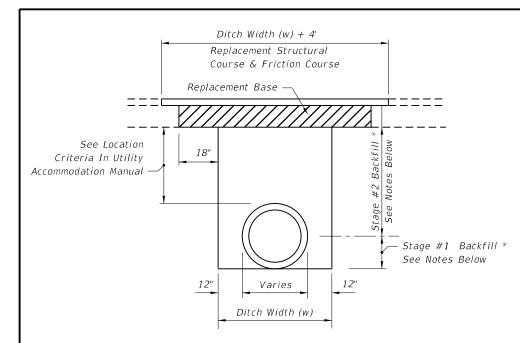
### GENERAL NOTES

1. The centerline of roadway and the centerline of bridge do not necessarily coincide. Prior to the placement of the expansion joint, the centerline of the roadway pavement shall be determined.

2. For information on other types of concrete pavement joints see Index No. 305.

3. Pay quantity for expansion joint is the length of joint to be constructed across the roadway and shoulder pavements, measured at right angles to the centerline of the roadway. Payment for expansion joint shall be full compensation for joint construction, including reinforced concrete subslab, sheet metal strip and compression seal, but, not including roadway pavement reconstruction associated with joint replacement or reconstruction. Expansion joint to be paid for under the contract unit price for Bridge Approach Expansion Joint, LF.

NSION JOINT	INDEX NO.	SHEET NO.
MENT	306	1



## FLEXIBLE PAVEMENT NOTES

### PAVEMENT REMOVAL AND REPLACEMENT

Pavement shall be mechanically sawed.

The replacement asphalt shall match the existing structural and friction courses for type and thickness in accordance with current FDOT asphalt mix specifications.

The new base materials shall be either of the same type and composition as the materials removed or of equal or greater structural adequacy (See Index No. 514).

### BACKFILL

### COMPACTED AND STABILIZED FILL OPTION

Backfill material shall be placed in accordance with Section 125 of the Standard Specifications.

In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.

In Stage #2, construct compacted fill along the sides of the pipe and up to the bottom of the base,with the upper 12" receiving Type B Stabilization. In lieu of Type B Stabilization, the Contractor may construct using Optional Base Group 3.

### \* FLOWABLE FILL OPTION

If compaction can not be achieved through normal mechanical methods then flowable fill may be used.

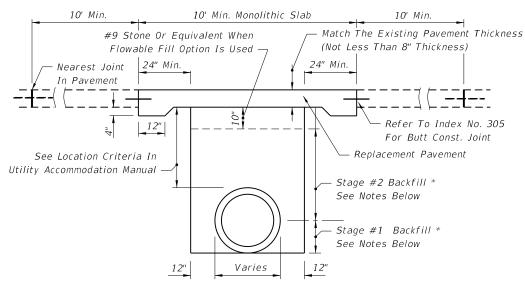
Flowable fill is to be placed in accordance with Section 121 of the Specifications, as approved by the Engineer.

Do not allow the utility being installed to float. If a method is provided to prevent flotation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.

In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.

In Stage #2, place flowable fill to the bottom of the existing base course.

### FLEXIBLE PAVEMENT CUT



## RIGID PAVEMENT NOTES

### PAVEMENT REMOVAL AND REPLACEMENT

High early strength cement concrete (3000 psi) meeting the requirements of Standard Specification 346 shall be used for rigid pavement replacement.

Pavement shall be mechanically sawed and restored to conform with existing pavement joints within 12 hours. (See Index No. 305)

### GRANULAR BACKFILL

Any edgedrain system that is removed shall be replaced with the same type materials. Any edgedrain system that is damaged shall be repaired with methods approved by the Engineer.

Fill material shall be placed in accordance with the Standard Specifications. Fill material shall be special select soil in accordance with Index No. 505.

In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.

In Stage #2, construct fill along the sides of the pipe and up to the bottom of replacement pavement.

### \* FLOWABLE FILL OPTION

If mechanical compaction can not be achieved through normal mechanical methods then flowable fill may be used.

Flowable fill is to be placed in accordance with Section 121 of the Specifications, as approved by the Engineer.

Do not allow the utility being installed to float. If a method is provided to prevent flotation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.

In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.

In Stage #2, place flowable fill to the bottom of the stone layer.

### RIGID PAVEMENT CUT

07/01/12

DESCRIPTION: LAST REVISION



TRENCH CUTS AND RESTORATIONS ACROSS ROADWAYS

2013

- selected.

MISCELLANEOUS UTILI

## GENERAL NOTES

1. The details provided in this standard index apply to cases in which jack and bore or directional boring methods are not required by the Engineer.

2. Flowable fill shall not be placed directly over loose, or high plastic, or muck material (see Index 505) which will cause settlement due to fill weight. Where highly compressible material exists, the amount, shape and depth of flowable fill must be engineered to prevent pavement settlement.

3. These details do not apply to utility cuts longitudinal to the centerline of the roadway which may require the additional use of geotextiles, special bedding and backfill, or other special requirements.

4. Method of construction must be approved by the Engineer.

5. Some pipe may require special granular backfill up to 6" above top of pipe. Geotextiles may be required to encapsulate the special granular material.

6. Where asphalt concrete overlays exist over full slab concrete pavement, the replacement pavement shall have an overlay constructed over the replacement slab. The overlay shall match the existing asphalt pavement thickness. The replacement friction course shall match the existing friction course, except structural course may be used in lieu of dense graded friction course.

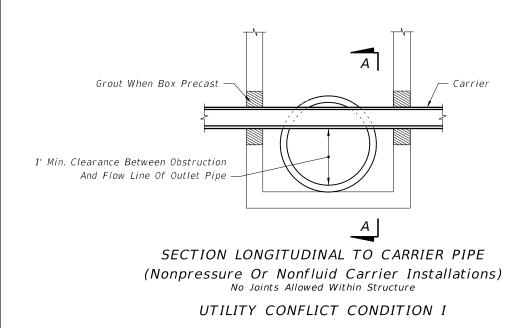
7. All shoulder pavement, curb, curb and gutter, and their substructure disturbed by utility trench cut construction shall be restored in kind.

8. The use of flowable fill to reduce the time traffic is taken off a facility is acceptable but must have prior approval by the Engineer. Flowable fill use is allowed only when properly engineered for pavement crossings, whether straight or diagonal, and shall not be installed for significant depths or lengths. The maximum length shall be fifty (50) feet and a maximum depth of six (6) feet unless supported by an engineering document prepared by a registered professional engineer that specializes in soils engineering. The engineering document shall address the evaluation of local groundwater flow interruption and settlement potential.

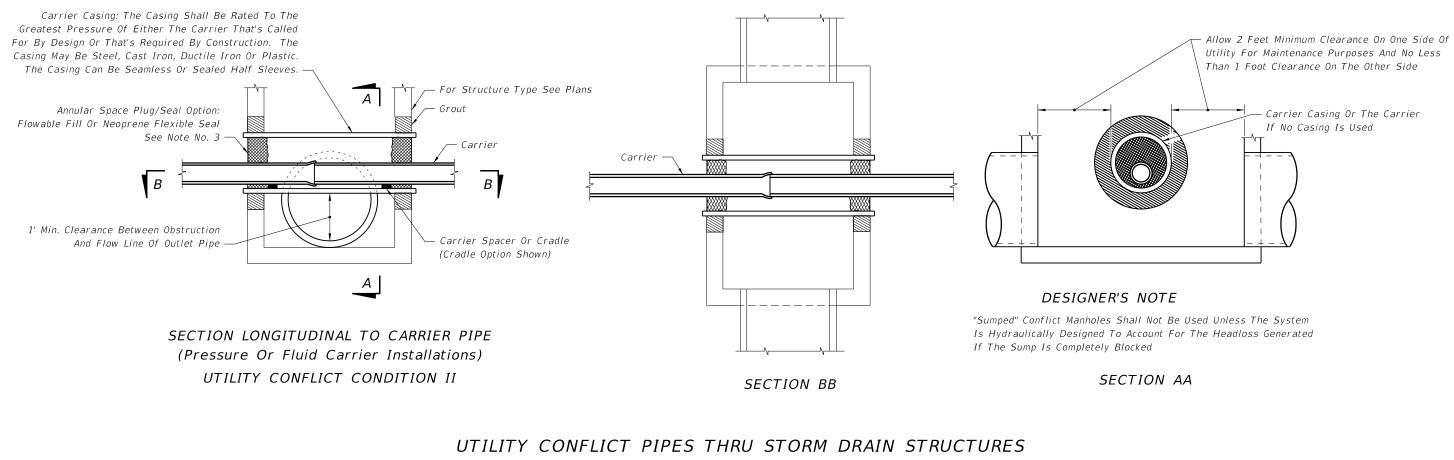
9. Excavatable flowable fill is to be used when the flowable fill option is

	INDEX	SHEET
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## NOTES FOR UTILITY CONFLICT PIPE

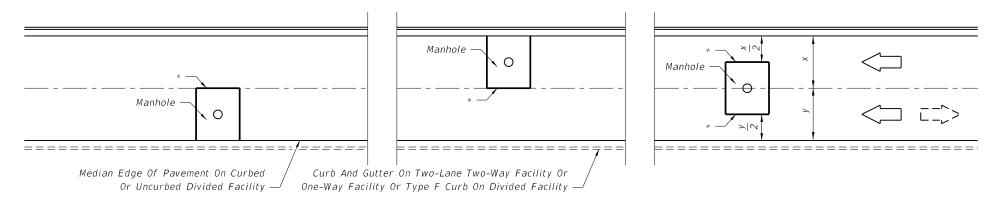


- 1. These details are for construction field expediency to resolve utility conflicts that cannot be remedied by relocation. For conflicts determined during design, use the construction shop drawings for structure details.
- 2. Concrete used in conflict structures shall be as specified in ASTM C478. 4000 psi may be used in lieu of Class I concrete.
- 3. Maximum opening for pipe shall be the pipe OD plus 6". Mortar used to seal the pipe into the opening will be of such mix that shrinkage will not cause leakage into or out of the structure.
- 4. If the conflict structure is round or there are multiple inlet or outlet pipes, then the wall section should be reviewed for strength.
- 5. If during construction or the plans design process it is determined that a potable water supply line must pass though a storm drain structure, it must be in compliance with Chapter 62-555.314 (3) F.A.C. and shown on the design or construction plans and submitted to the Florida Department of Environmental Protection (FDEP) Administrator For Drinking Water in the respective FDEP District for review and comment. This index and rule citation provide accepted methods for addressing conflicts when and where they cannot be reasonably avoided. To be submitted along with the plans shall be a justification describing inordinate cost and the impracticality of avoidance. If identified, properly justified, and accomplished in accordance with this index, approval is granted. Upon request, the Utility Agency Owner (UAO) must provide support data on the cost of relocation or adjustment to the FDOT for submittal to the FDEP. See the following web site for District FDEP Drinking Water Contacts: www.dep.state.fl.us/water/drinkingwater/index.htm and click on "Organization" on the menu to the right.



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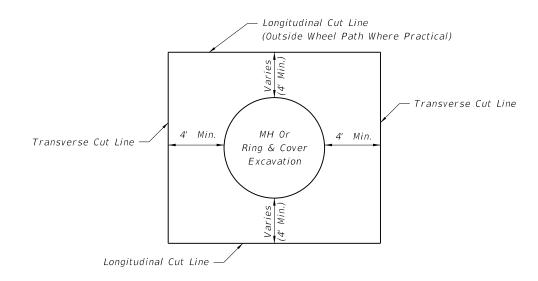
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\* Longitudinal Cut Lines For Both Curbed And Uncurbed Facilities Must Coincide With A Regular Seam Or Midlane Point In Order To Be Outside The Wheel Path







PARTIAL CUTS FOR RING AND COVER ADJUSTMENTS

### NOTES

- 1. No irregular seams are permitted. All seams must be clean sawed.
- 2. Pavement cut seams for underground utility structures in rigid pavement are the same longitudinally, but the transverse seams shall extend to the nearest existing joint.

3. See Sheet 1 for replacement pavement.

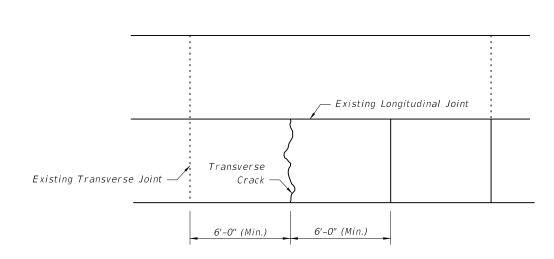
# NONTRENCH PAVEMENT CUTS FOR UNDERGROUND UTILITY STRUCTURES IN PAVEMENT

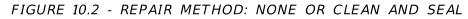
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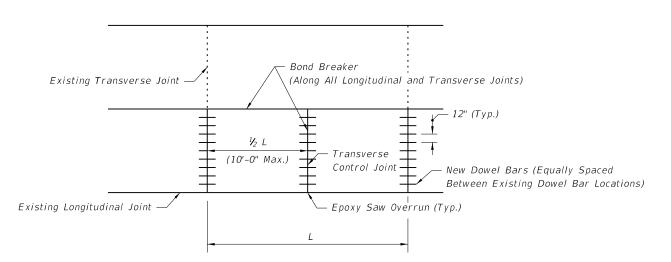


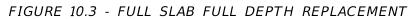
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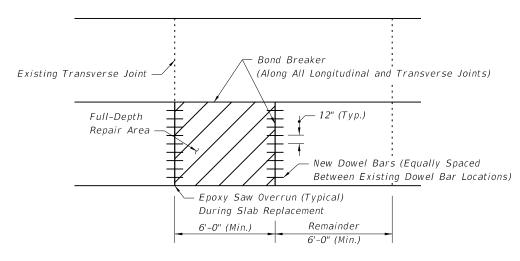
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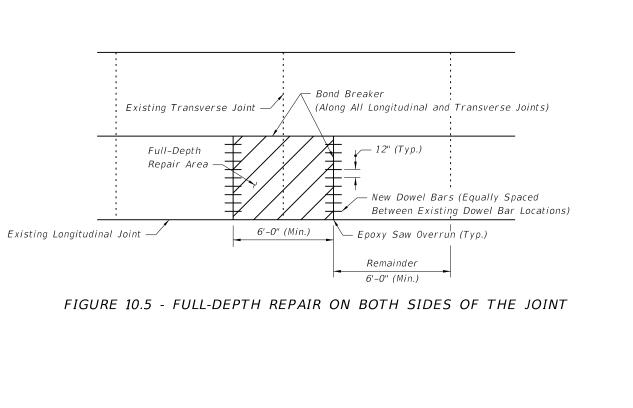


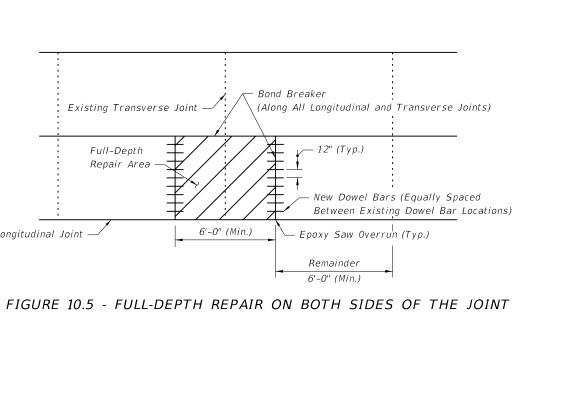


# FIGURE 10.4 - PARTIAL SLAB FULL DEPTH REPLACEMENT









### GENERAL NOTES

- 1. For Repair and Replacement Criteria see Sheet 2.
- slab to the bottom of the concrete.
- to penetrate more than 0.5 in. into the base.
- grade.
- replaced by the contractor at his expense.
- first.
- ероху.

# CONCRETE SLAB REPL

2. Full depth repairs consist of removing and replacing at least a portion of the existing

3. Repair boundaries shall be sawed full-depth with diamond saw blades. On hot days, it may not be possible to make this cut without first making a wide, pressure relief cut within the repair boundaries. A carbide-tipped wheel saw may be used for this purpose, but the wheel saw must not intrude on the adjacent lane, unless the lane is slated for repair. The wheel saw cuts produce a ragged edge that promotes excessive spalling along joints. Hence, if wheel saw cuts are made, diamond saw cuts must be made 18 in. outside the wheel saw cuts. To prevent damage to the base, the wheel saw must not be allowed

4. No additional base or subgrade material shall be added and all loose base or subgrade material shall be removed prior to placement of the new concrete slab. The concrete slab shall be placed to the full depth of the material removed. No additional compensation will be allowed for additional concrete required to bring proposed concrete slab up to finished

5. Removal of the damaged concrete pavement shall be by lifting. Any good concrete pavement which is damaged during removal of damaged areas shall be removed and

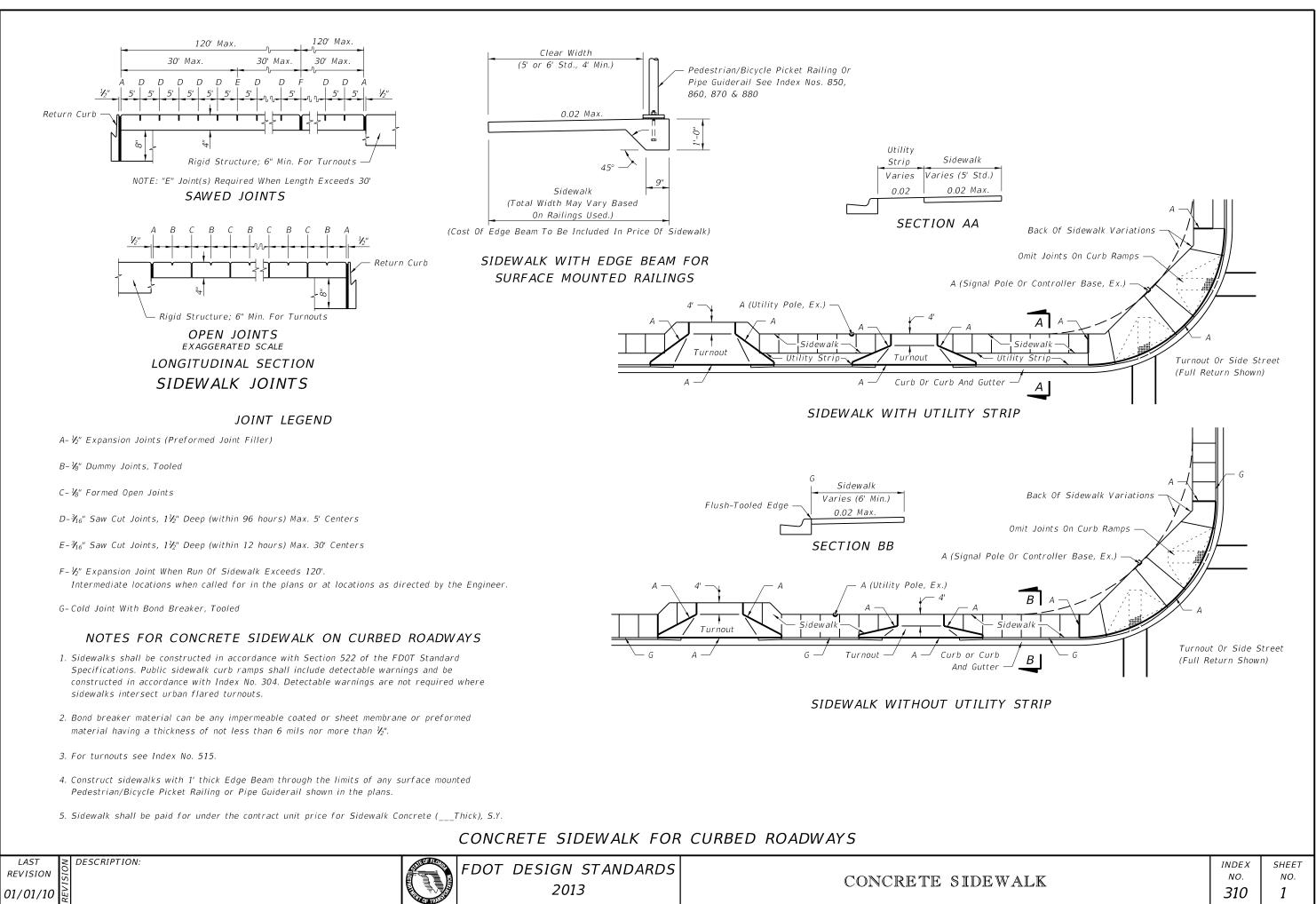
6. If the roadway contract includes grinding, then the slab replacement shall be performed

7. During slab replacement operations, fill any saw cut over runs into adjacent slabs with

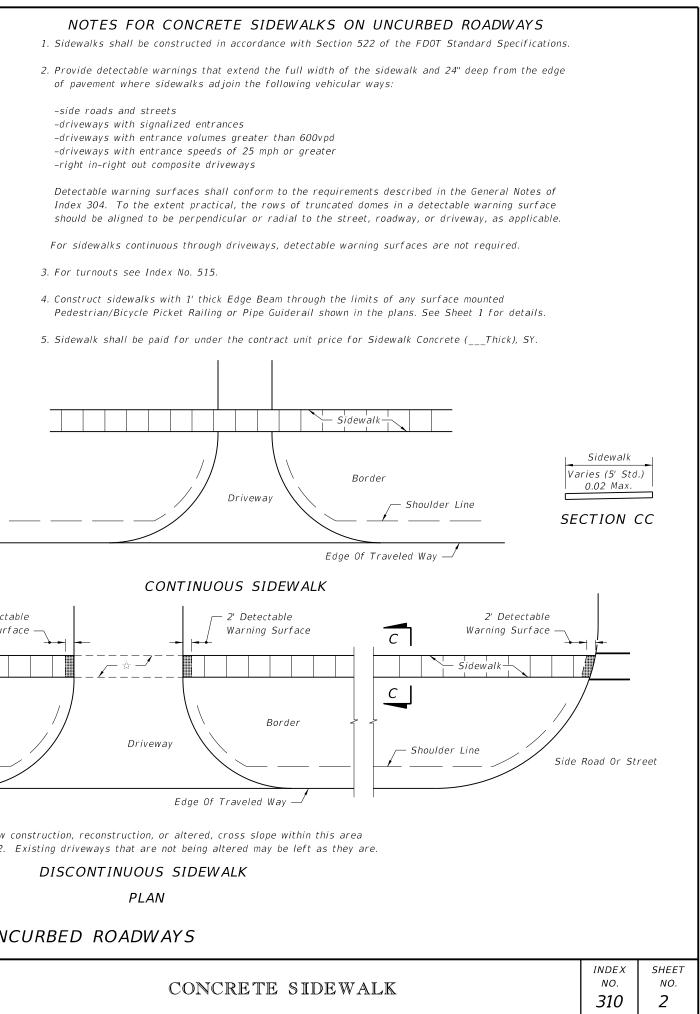
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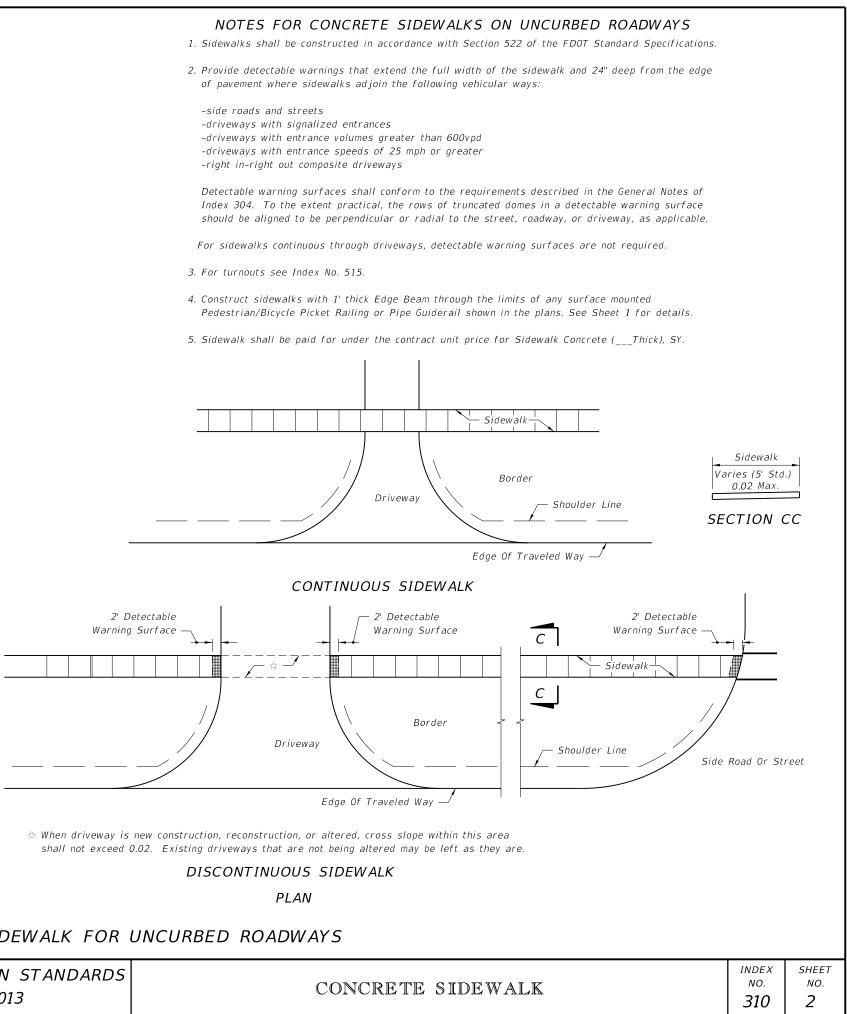
		SEVERITY/DESCRIPTION	REPAIR METHOD	REFERENCE	
CRACKING					
	Light	$<\!$	None	Figure 10.2	
Longitudinal	Moderate	$V_8^{"}$ <width <<math="">V_2^{"}, spalling &lt;3" wide</width>	Clean and Seal	Figure 10.2	
	Severe	width > $V_2$ ", spalling >3" faulting > $V_2$ "	Replace	Figure 10.3	
	Light	$<\!$	None	Figure 10.2	
Transverse	Moderate	$V_8$ " <width <<math="">V_2", spalling &lt;3" wide</width>	Clean and Seal		
	Severe	width > $V_2$ ", spalling >3" faulting > $V_2$ "	Replace	Figure 10.3, 10.4 and 10.5	
Corner Breaks	adjacent lo	<sup>t</sup> the slab is separated by a crack that intersects the ngitudinal and transverse joint, describing an approximate ith the direction of traffic.	Full Depth	Figure 10.4 and 10.5	
Intersecting Random Cracks (Shattered Slab)	Cracking pa	atterns that divide the slab into three or more segments.	Full Depth	Figure 10.3 and 10.4	
JOINT DEFICIENCIES					
	Light	spall width <1 $\mathcal{V}_2$ ", < $\mathcal{V}_3$ slab depth, <12" in length	None	Figure 10.4 and 10.5	
Spall Nonwheel Path	Moderate	$1 \mathcal{V}_2$ " <spall <="" <3",="" <math="" width="">\mathcal{V}_3 slab depth, &lt;12" in length</spall>	None	Figure 10.4 and 10.5	
	Severe	spall width >3" or length >12"	Full Depth	Figure 10.4 and 10.5	
	Light	spall width <1½", <than <math="">rac{1}{3} slab depth, &lt;12" in length</than>	None	Figure 10.4 and 10.5	
Spall Wheel Path	Moderate	$1\frac{1}{2}$ " <spall <="" <3",="" <math="" width="">\frac{1}{3} slab depth, &lt;12" in length</spall>	Full Depth	Figure 10.4 and 10.5	
	Severe	spall width >3" or length >12"	Full Depth	Figure 10.4 and 10.5	
RFACE DETERIORATION	'				
Pop Outs Nonwheel Path		is of surface pavement broken loose, normally ranging 4 in. diameter and $\mathcal{V}_2$ to 2 in. in depth.			
,	Light	Not deemed to be a traffic hazard	Keep under observation		
	Severe	Flying debris deemed a traffic hazard	Full Depth	Figure 10.4	
Pop Outs Wheel Path		es of surface pavement broken loose, normally er and 2" in depth.			
	Light	Deemed to be a traffic hazard	Full Depth	Figure 10.4	
	Severe	Flying debris deemed a traffic hazard	Full Depth	Figure 10.4	
CELLANEOUS DISTRES.	5				
	Elevation d	ifferences across joints or cracks.			
Faulting	Light	Faulting <4/32"	None		
	Moderate	4 <faulting 32"<="" <16="" td=""><td>Grind</td><td>1</td></faulting>	Grind	1	
	Severe	Faulting >16/32"	Grind		
	Light	0 <drop-off <1"<="" td=""><td>None</td><td></td></drop-off>	None		
Lane To Shoulder Drop-Off	Moderate	1" <drop-off <3"<="" td=""><td>Build Up</td><td>N/A</td></drop-off>	Build Up	N/A	
	Severe	drop-off >3 "	Build Up	1	
Water Bleeding Or Pumping	Seeping or ejection of water through joints or cracks.		Install appropriate drainage, edge drain, permeable subbase, reseal joints, etc.	N/A	
		vement at transverse joints or cracks often d by shattering of the concrete.	Full Depth	Figure 10.3 and 10.4	

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	FDOT DESIGN STANDARDS 2013	CONCRETE	SIDEWA

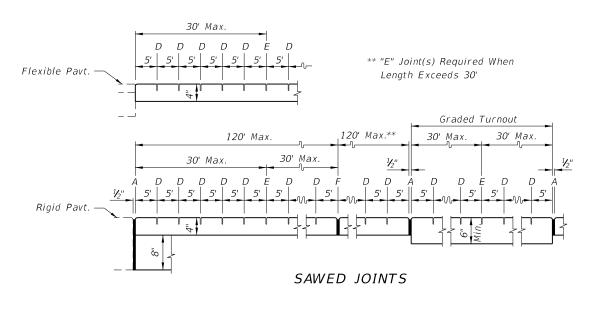


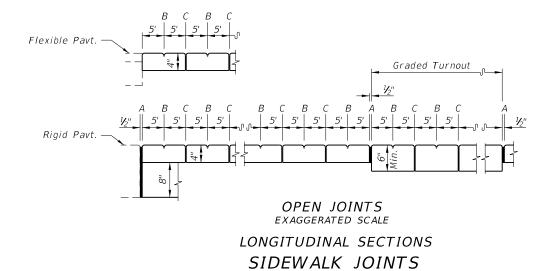


# CONCRETE SIDEWALK FOR UNCURBED ROADWAYS

FDOT	DESIGN	STANDARDS	
	201	STANDARDS 3	

NEOF





# JOINT LEGEND

A-  $\frac{1}{2}$ " Expansion Joints (Preformed Joint Filler)

- B-1/8" Dummy Joints, Tooled
- C-∛16" Formed Open Joints
- $D-\mathscr{Y}_{16}$ " Saw Cut Joints,  $1\mathscr{Y}_2$ " Deep (96 Hour) Max. 5' Centers
- $E-\frac{1}{2}$ " Saw Cut Joints,  $1\frac{1}{2}$ " Deep (12 Hour) Max. 30' Centers
- $F \frac{1}{2}$ " Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.

DESCRIPTION:

LAST	N	Ľ
REVISION	SIC	
07/01/09	REVI	