### GENERAL NOTES

- 1. The illustrations for guardrail applications are standard configurations; adjustments are to be made as required by site specific conditions to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figures 1 and 2, and other application details of this Index.
- 3. One Panel (i.e., panel length) equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standards (indexes) or specifically called for in the plans.

Post spacing shall be 6'-3" except that reduced spacing shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Detail J and Index No. 402) and transitions to redirective crash cushions, (b) the conditions in Note No. 7 below, (c) special post applications, (d) reduced post spacing required for specific end anchorage assemblies, and, (e) specific spacing called for in the plans.

- 4. Guardrail mounting height for the W-beam without rubrail and for thrie-beam is 1'-9" to the center of beam, and for W-beam with rubrail 2'-0" to center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a tolerance of 3" above and 1" below the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines). For guardrail placed on slopes beyond the shoulder point, there shall be no deviation more than 1" below to 3" above the desired height within any 25 foot section of guardrail.
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end treatments for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end treatments will be constructed only when restraints prevent construction of flared end anchorages.

Guardrail end anchorage assemblies shall be of the type called for in the plans. If the plans call for end anchorage assembly "flared" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDOT approved flared assembly provided in this Index or identified on the Qualified Products List (QPL), subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for end anchorage assembly "parallel" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDOT approved parallel assembly provided in this Index or identified on the QPL, subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for a specific end anchorage assembly, substitutions with other end anchorage assemblies will not be permitted unless approved by the Engineer. Approved substitutions will not be eligible for CSIP consideration.

When an end treatment is attached to guardrail with Pedestrian Safety Treatment, only end treatment systems with timber posts are to be used.

Existing approved proprietary end anchorage systems are identified on the Qualified Products List (QPL). After January 1, 2011 manufacturers seeking approval of new proprietary end anchorage systems for inclusion on the QPL must submit application along with design documentation showing the end anchorage system; is crash tested to Test Level 3 criteria in accordance with the Manual for Assessing Safety Hardware (MASH), is accepted by FHWA for use as a guardrail end anchorage system, and is compatible with FDOT guardrail systems. System approvals will be contingent on FDOT's evaluation of crash test performance results for consistency with FDOT guardrail application and use. If approved, installation drawings signed and sealed by a professional engineer licensed in the State of Florida will be required.

- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 4' minimum for standard W-beam, other guardrail configurations with reduced post spacing may be applicable; see General Note No. 11 and the minimum offset table on Sheet 19. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier wall shall be used unless other shielding is approved by the Engineer of Record. See Index No. 410 for safety shape concrete barrier walls and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at roadside hazards or other areas where the Engineer has deemed guardrail necessary, guardrail should be considered on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone and fill heights are 6' or greater. Curbed sections where fill slopes are steeper than 1:3 and fill heights are 6' or greater within 22' of the traveled way should be evaluated for installation of guardrail. Additional guidance for evaluating the need for guardrail can be found in the Plans Preparation Manual.
- 9. The guardrail to bridge connections contained in this Index are for bridges with Test Level 4 traffic railing barriers. For guardrail to concrete barrier wall connections see Index No. 410. For existing bridges receiving retrofit traffic railing barriers see Index No. 402.

FDOT	DES	SIGN	STANDARDS 7/2013
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- 10. The W-beam guardrail system in this index is the standard system to be used on the State Highway System where a Test Level 3 semi-rigid barrier is required.
- 11. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic railing barriers, to concrete and certain water filled safety shaped barriers, certain crash cushions and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 402, 410 and 414. The use of thrie-beam guardrail with standard offset blocks (Test Level 3 semi-rigid system) may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
  - a W-beam deflection is marginal
- b. W-beam with rubrail considered functionally deficient,
- c. Vehicle overriding W-beam is probable,
- e. High frequency of repairs to W-beam,
- f. Spandrel beam with low deflection needed around unrelocatable structure,

The modified thrie-beam guardrail is a Test Level 4 semi-rigid system and may be used where a Test Level 4 guardrail is required.

- 12. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
- a. Wide medians where approach end anchor is located outside of opposing roadway clear zone,
- b. Medians of uniform width that are occupied by other transportation and joint use facilities,
- installations,
- d. Medians of bifurcated roadways.
- 13. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 14. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crashworthiness at terminals. Crash cushions shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 15. Corrugated sheet steel beams, end shoes, end sections and backup plates shall conform to the current requirements of AASHTO M180, Class A, Type II (zinc) coating. All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTO requirements.
- 16. Offset blocks:
  - a. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing COMBINATIONS are tabulated on Sheet 16.
  - b. Plastic offset block installations shall be constructed on guardrail outside of approach end anchorage assemblies or any transition system connecting to a rigid or thrie-beam barrier.
- 17. Where necessary to enlarge or add holes to galvanized guardrail, the work will be done by drilling or reaming. Damaged galvanized guardrail will be metalized in accordance with Sections 562 and 971 of the Standard Specifications. No burning of holes will be permitted.
- 18. For guardrail reflector details see Sheet 17.
- 19. Any run of guardrail with existing concrete posts that is being reset under a construction or maintenance contract shall be reset using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 20. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for CSIP consideration.
- 21. On roadways designated for reverse laning, all downstream ends of guardrail that are not shielded or that are not designed as approach end terminals shall be marked with post-mounted Type 3 Object Markers. Trailing bridge ends and trailing shoulder concrete barrier wall ends shall be marked with Type 3 Object Markers except where there is trailing end guardrail. Object markers to be installed facing reverse laning traffic. The cost of the object marker shall be included in the cost of the guardrail.

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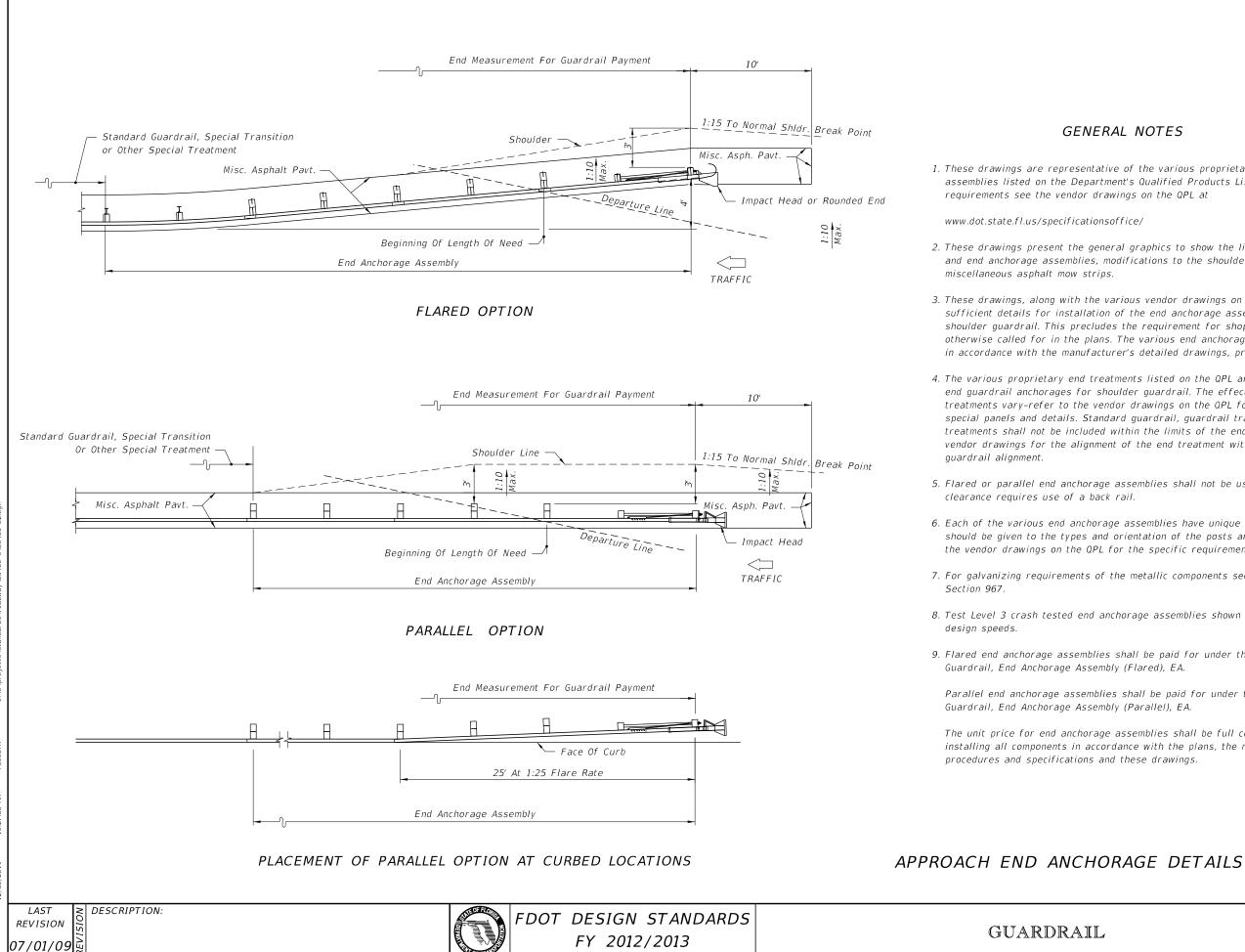
d. Drainage will be impeded or blocked by the use of concrete barrier wall (subject to deflection space requirements),

g. Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g., passenger vans and small buses).

c. Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail

steel offset blocks may remain throughout the service life of the existing guardrail. PERMISSIBLE POST AND OFFSET BLOCK

	INDEX	SHEET
GUARDRAIL	NO.	NO.
	400	1



# GENERAL NOTES

1. These drawings are representative of the various proprietary guardrail end anchorage assemblies listed on the Department's Qualified Products List (QPL). For specific details and

2. These drawings present the general graphics to show the limits of payment for guardrail and end anchorage assemblies, modifications to the shoulder and placement of the

3. These drawings, along with the various vendor drawings on the QPL, are intended to show sufficient details for installation of the end anchorage assemblies and their connection to shoulder guardrail. This precludes the requirement for shop drawing submittals unless otherwise called for in the plans. The various end anchorage assemblies shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.

4. The various proprietary end treatments listed on the QPL are intended for use as approach end guardrail anchorages for shoulder guardrail. The effective length of the end treatments vary-refer to the vendor drawings on the QPL for the length and the use of special panels and details. Standard guardrail, guardrail transitions or other special treatments shall not be included within the limits of the end anchorage assembly. See the vendor drawings for the alignment of the end treatment with respect to the normal

5. Flared or parallel end anchorage assemblies shall not be used in medians where horizontal

6. Each of the various end anchorage assemblies have unique features. Careful attention should be given to the types and orientation of the posts and other components. Refer to the vendor drawings on the QPL for the specific requirements of each system.

7. For galvanizing requirements of the metallic components see Standard Specifications

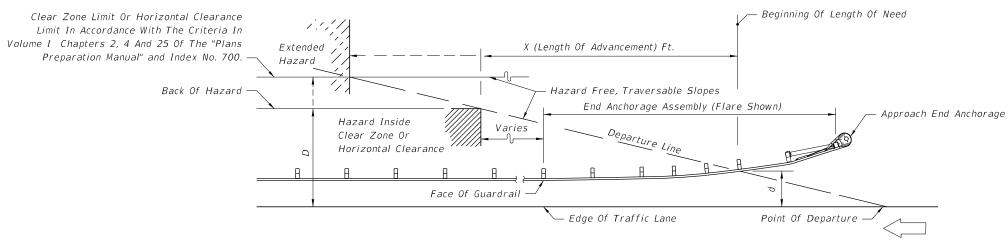
8. Test Level 3 crash tested end anchorage assemblies shown on the QPL are suitable for all

9. Flared end anchorage assemblies shall be paid for under the contract unit price for

Parallel end anchorage assemblies shall be paid for under the contract unit price for

The unit price for end anchorage assemblies shall be full compensation for furnishing and installing all components in accordance with the plans, the manufacturer's detail drawings,

	INDEX NO.	SHEET NO.
,	400	2



Design Speed mph	X (Length Of Advancement) Ft. (See NOTES 1 & 2)
<u>≤</u> 45	= 16 (D-d)
≥50	= 13 (D-d)

### NOTES

- 1. Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.
- 2. The flared end anchorage with 4' nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

### Equation Variables:

D = Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

d = Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

For flared and parallel end anchorage assemblies the beginning length of need is to be set at the center of post #3. That is, the departure line must intersect the face of the rail at post #3.

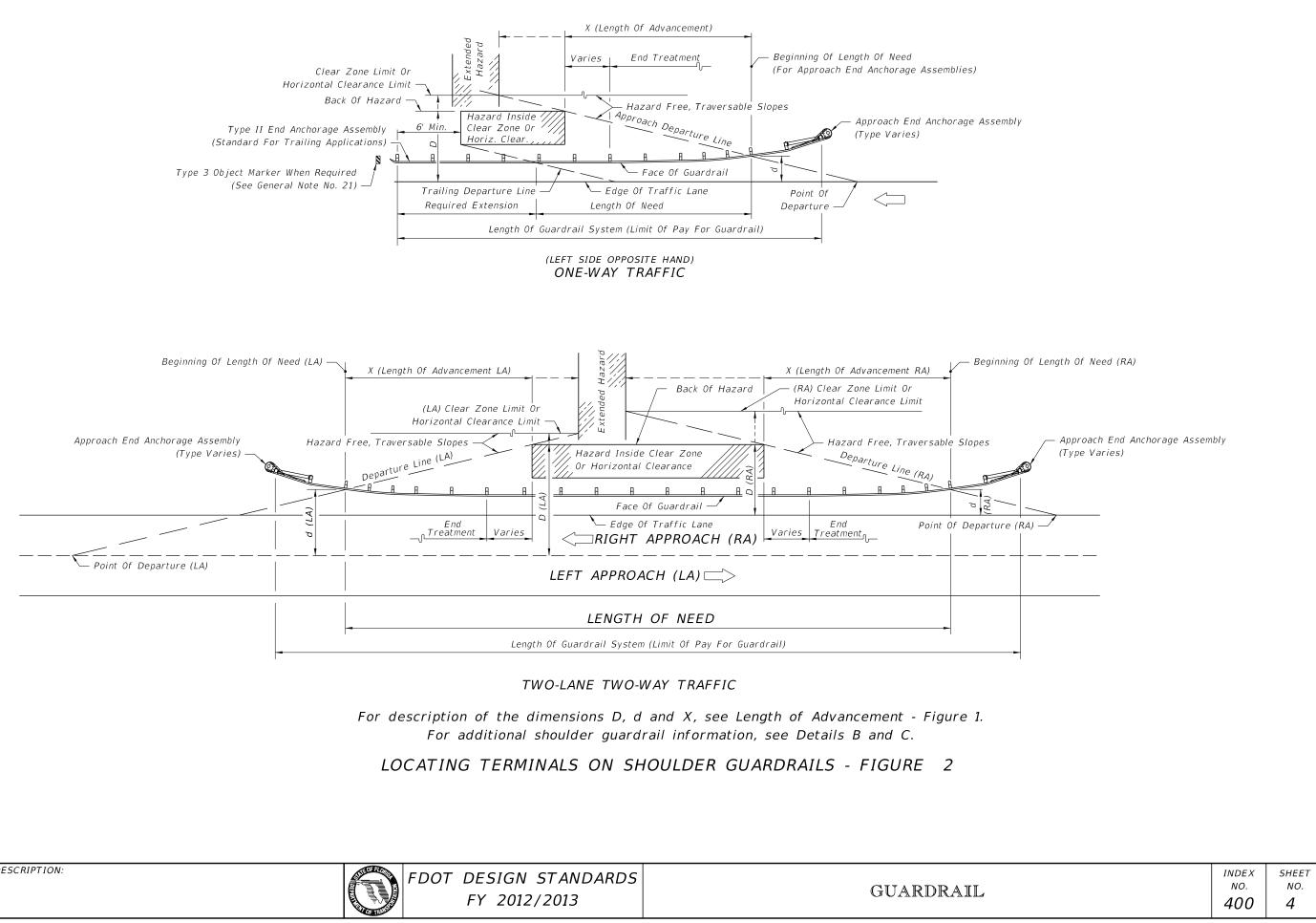
For flared end anchorage assemblies the offset distance "d" will equal the normal guardrail offset measured from the face of the guardrail to the edge of the near approach travel lane plus 1'-2" for 45 mph or less and 1'-91/4" for greater than 45 mph.

# LENGTH OF ADVANCEMENT - FIGURE 1

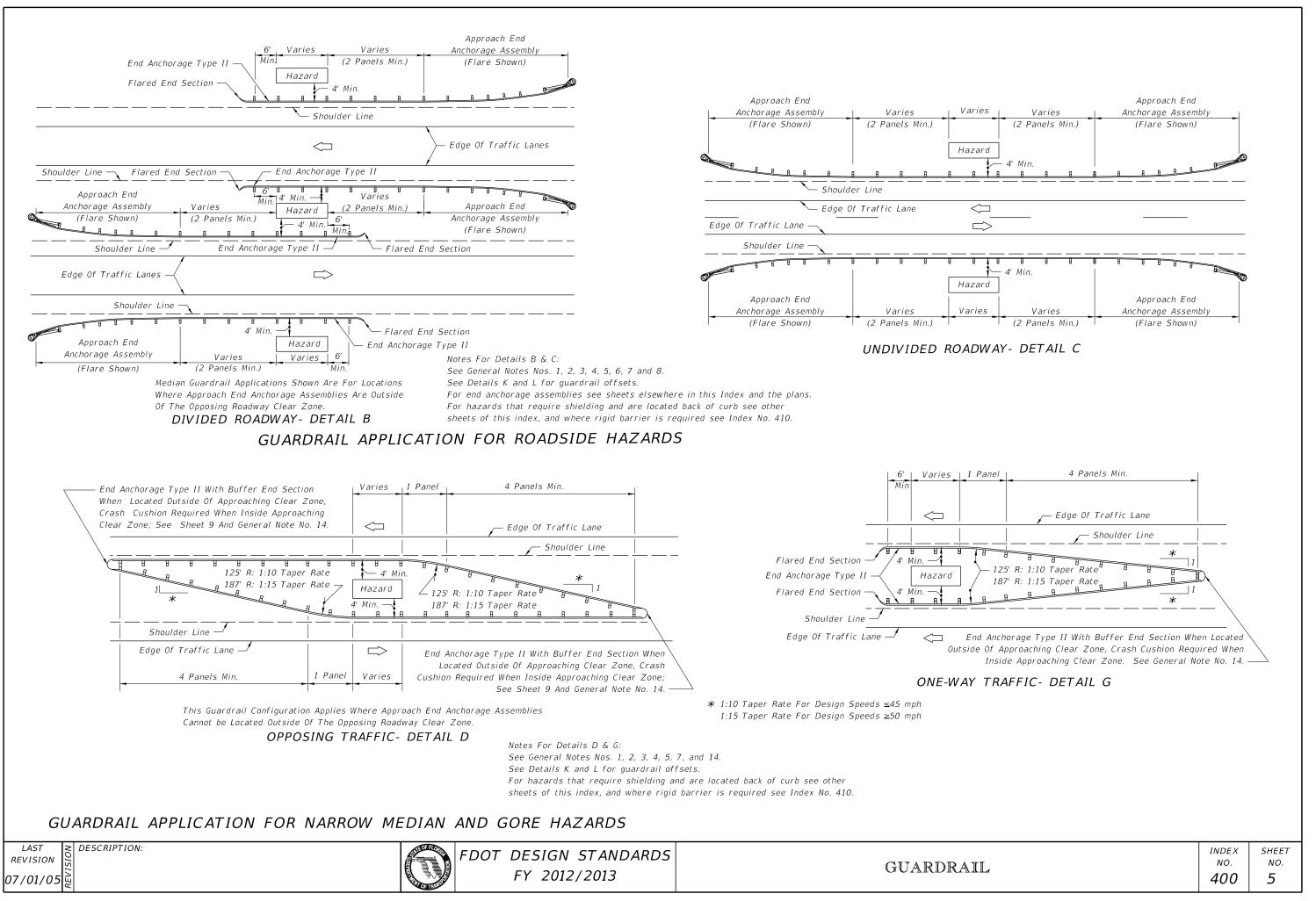


FDOT DESIGN STANDARDS FY 2012/2013

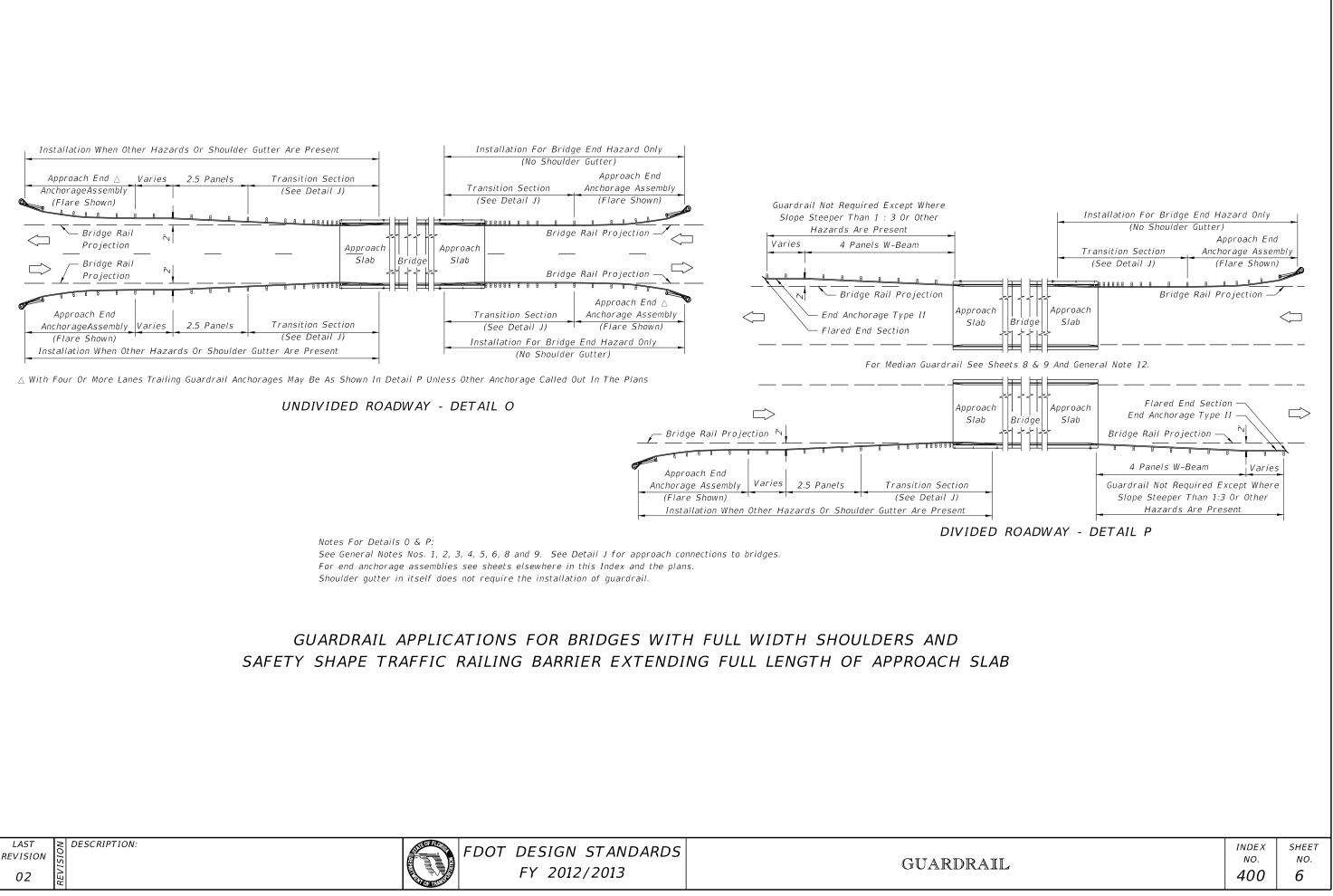
INDEX NO.	SHEET NO.
400	3.



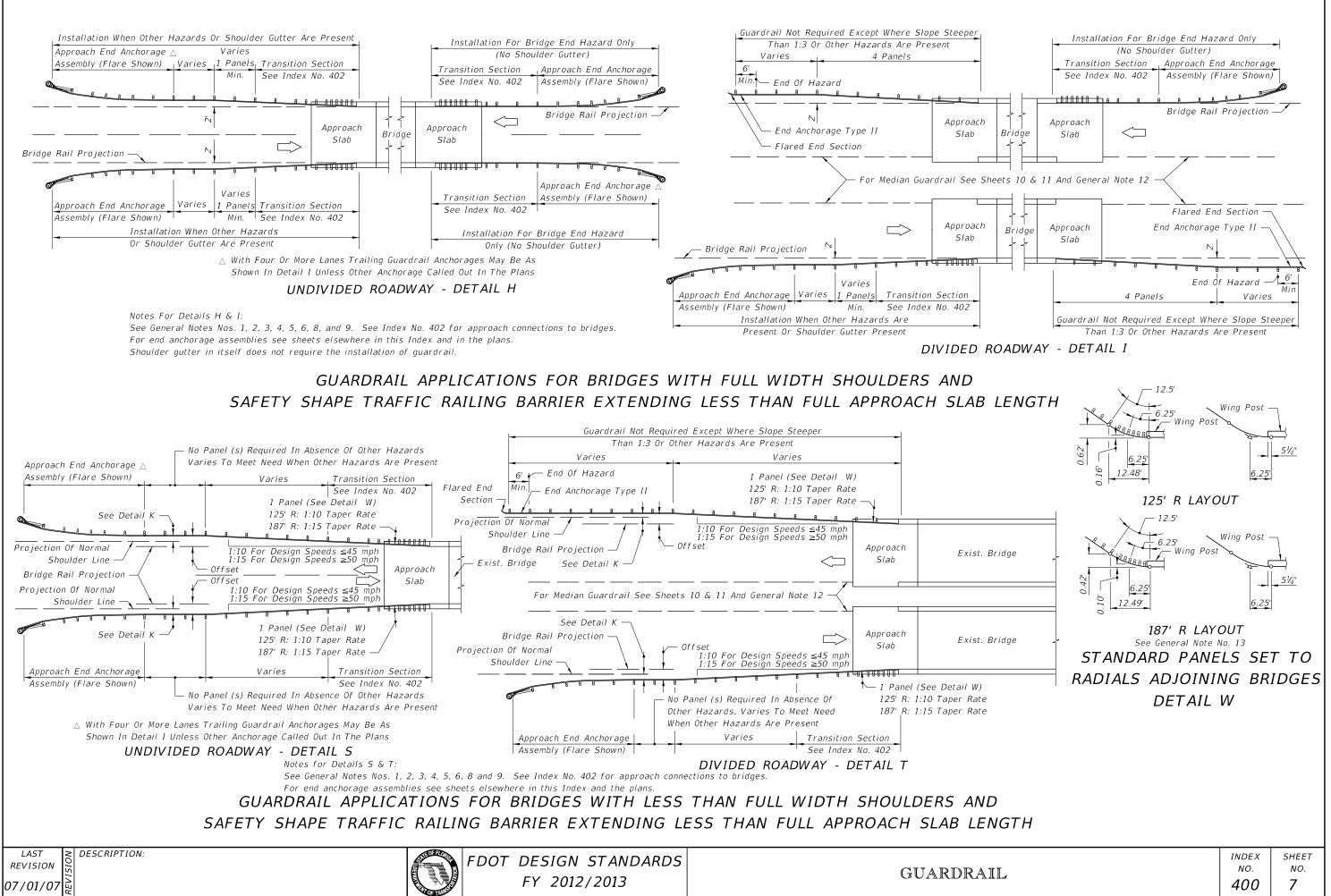


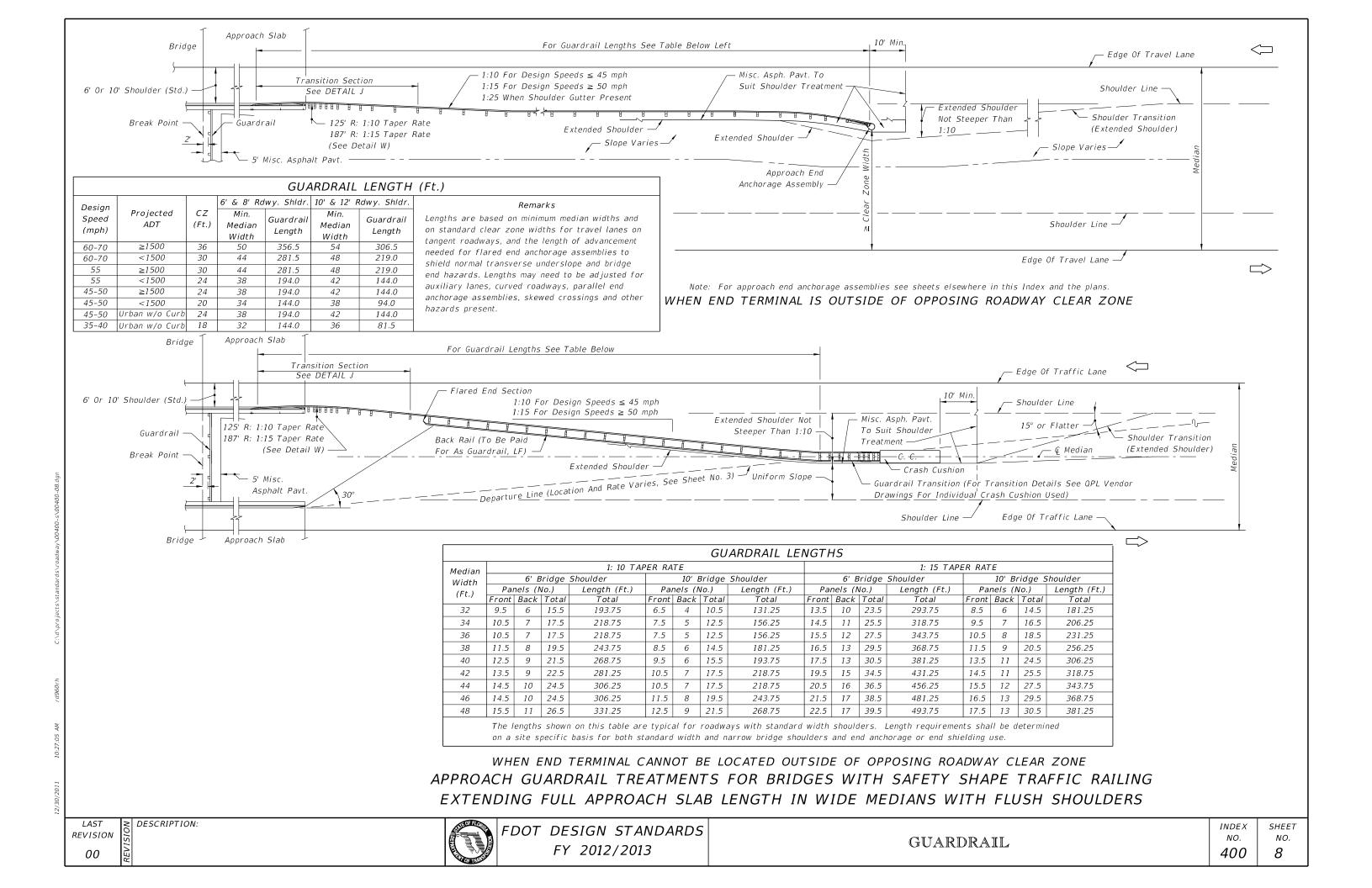


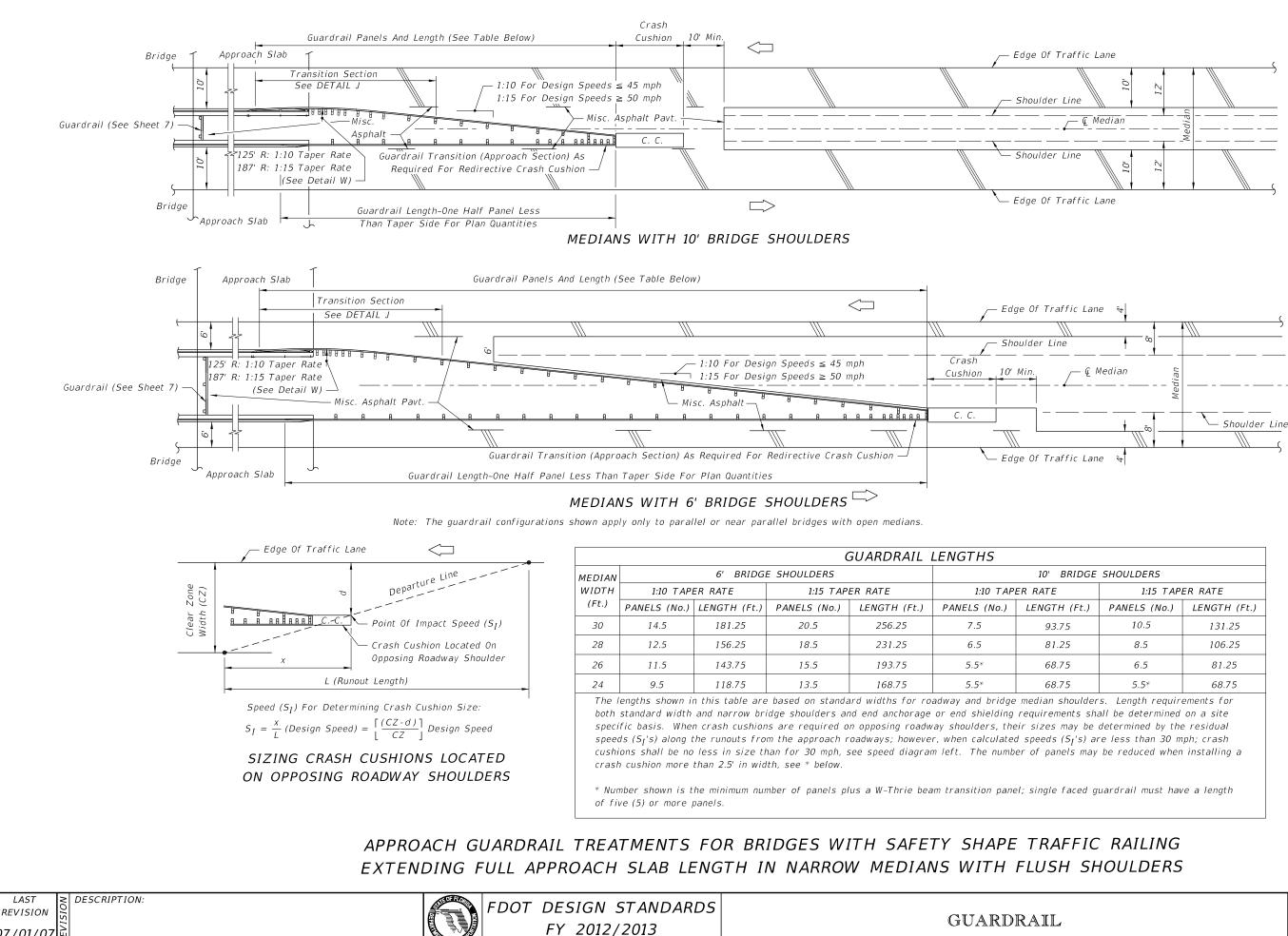
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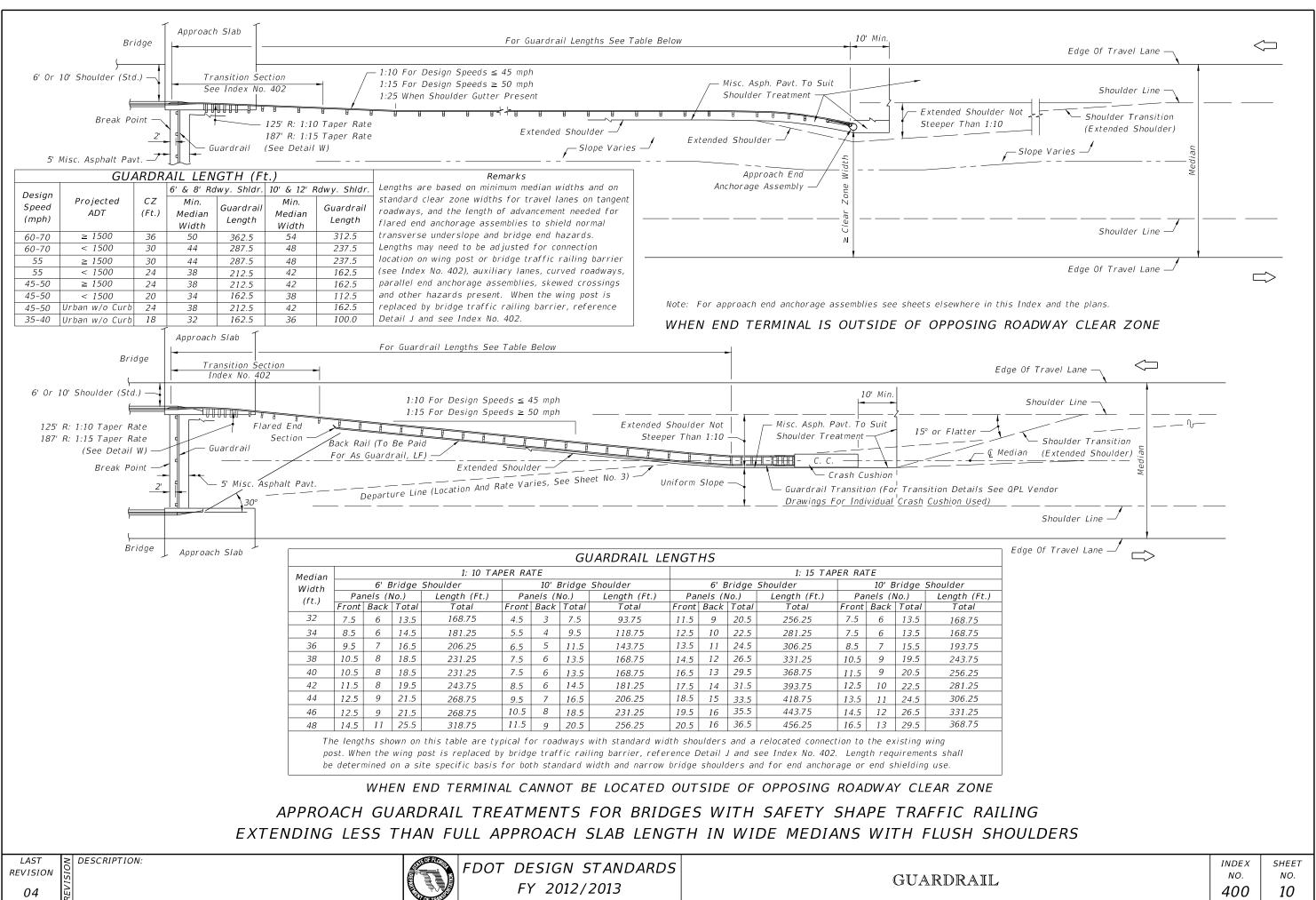




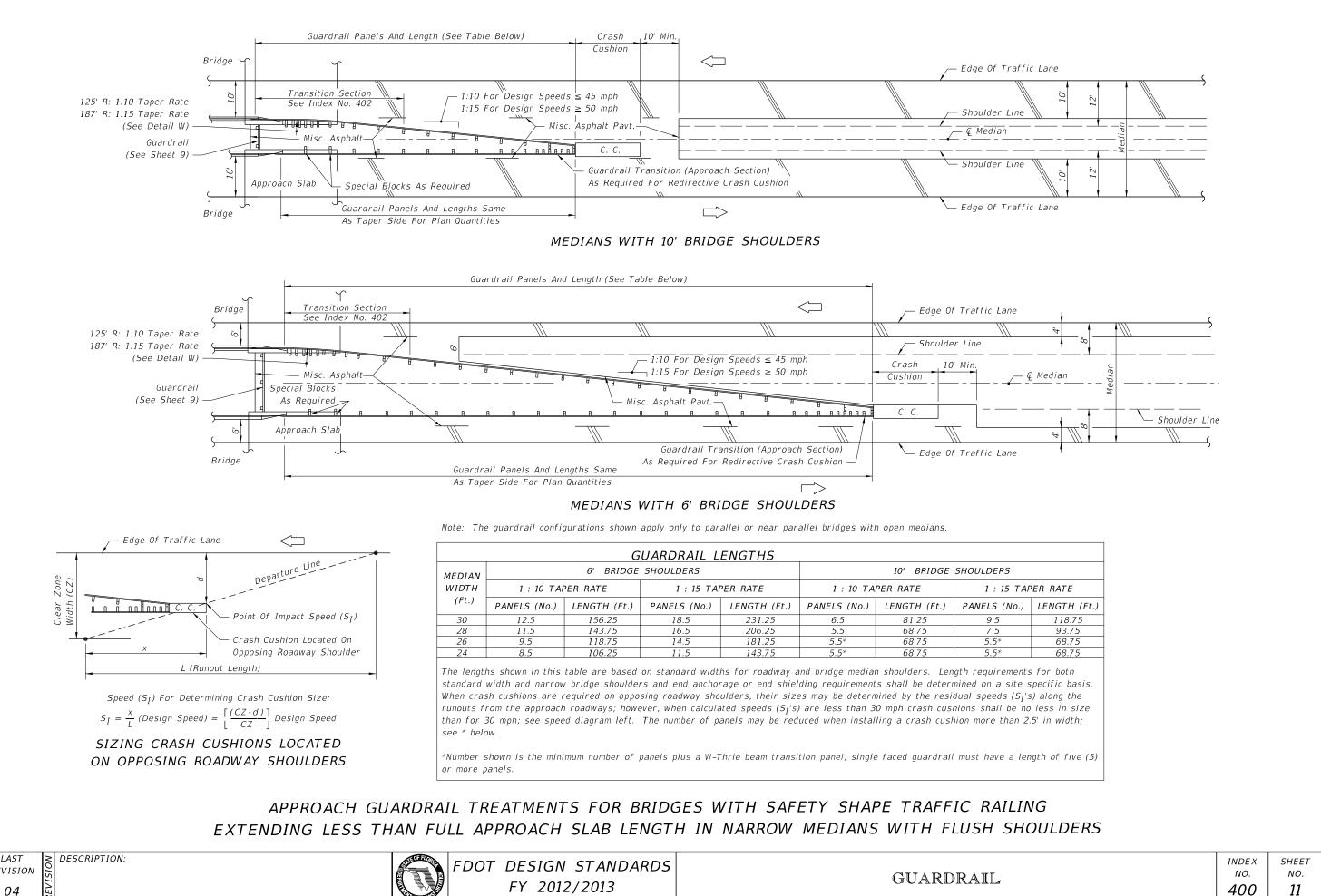
07/01/07

BRIDGE SHOULDERS					
	1:15 TAPER RATE				
⊣ (Ft.)	PANELS (No.)	LENGTH (Ft.)			
75	10.5	131.25			
25	8.5	106.25			
75	6.5	81.25			
75	5.5*	68.75			

INDEX NO.	SHEET NO.
400	9



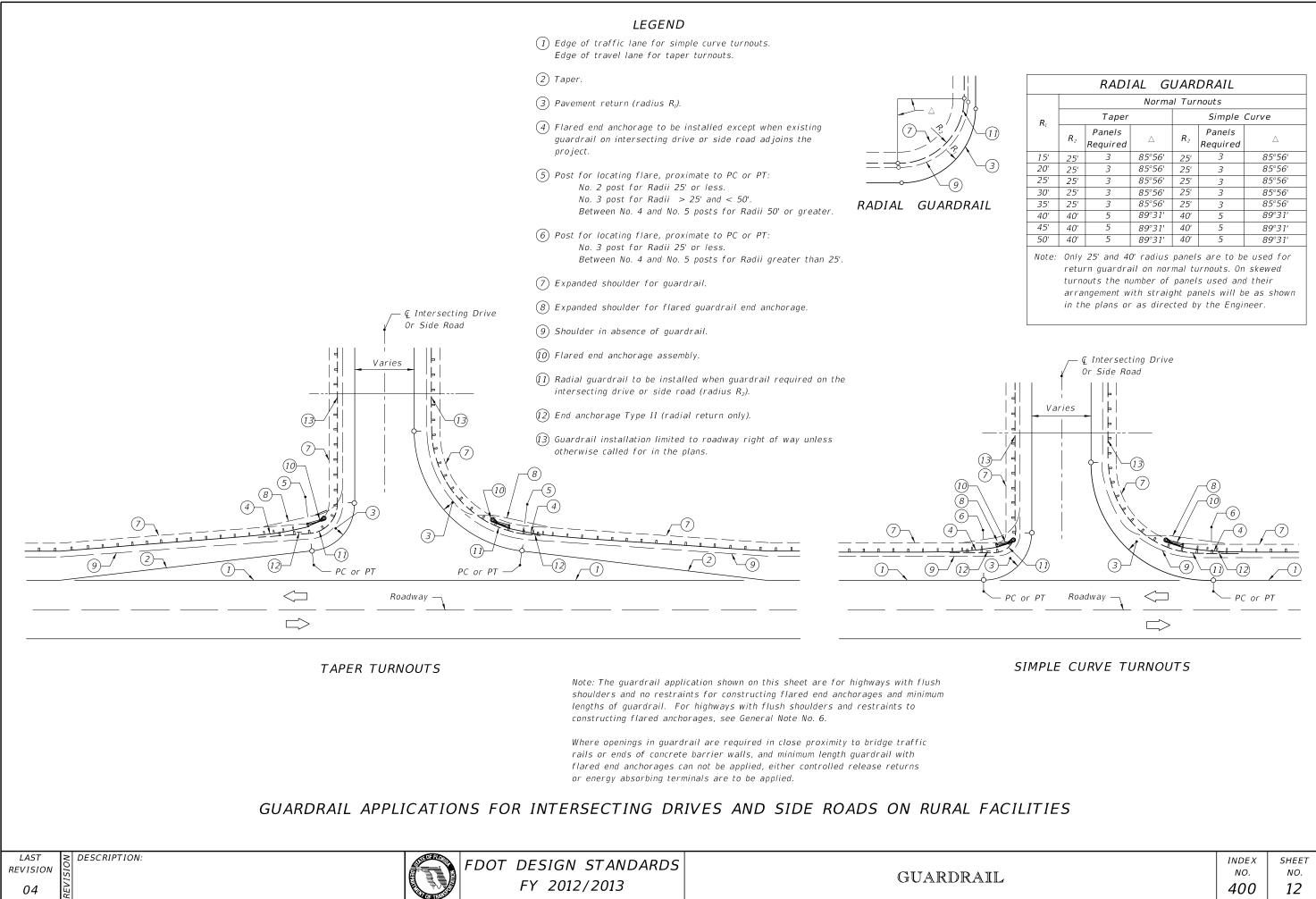
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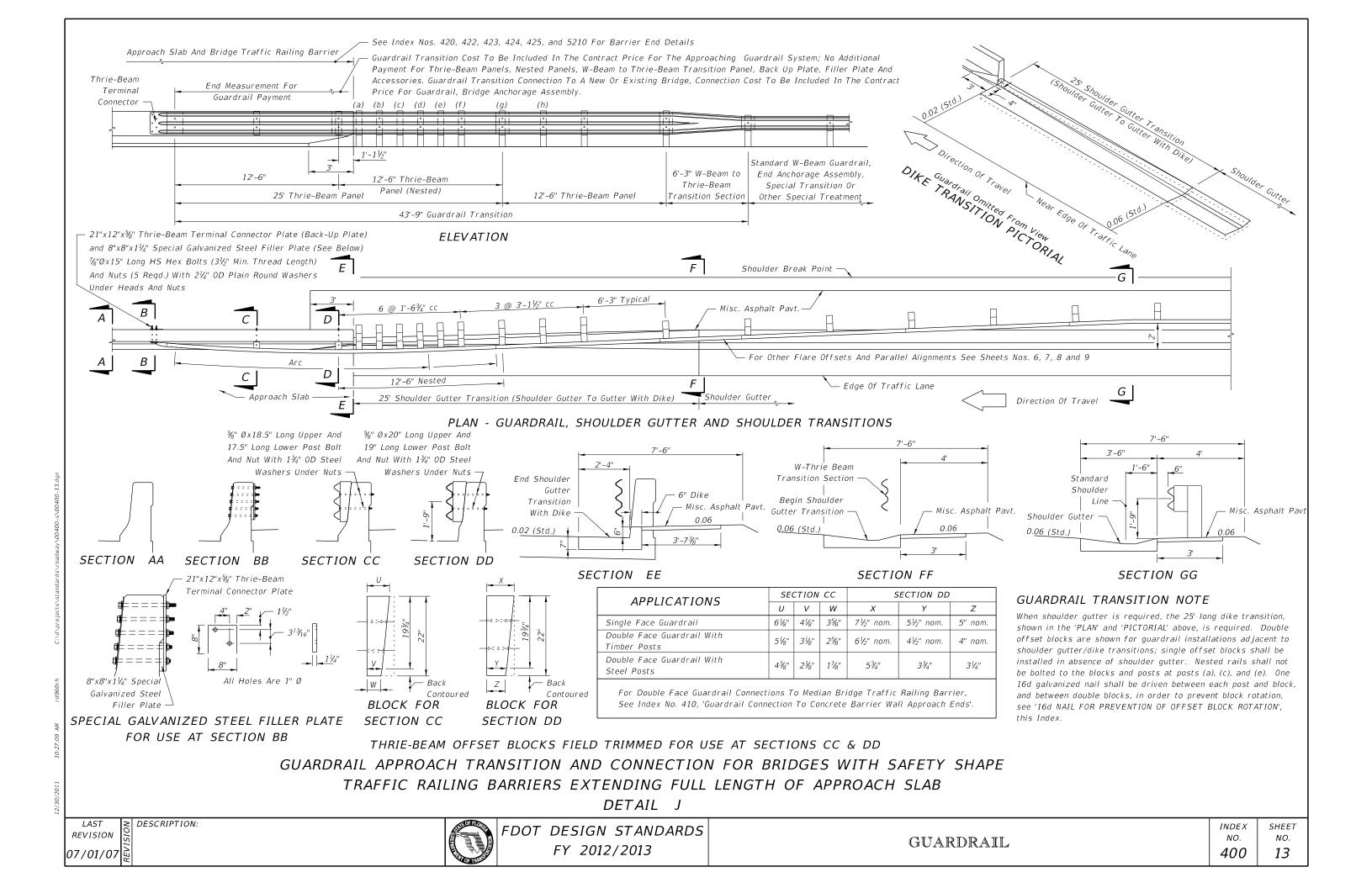
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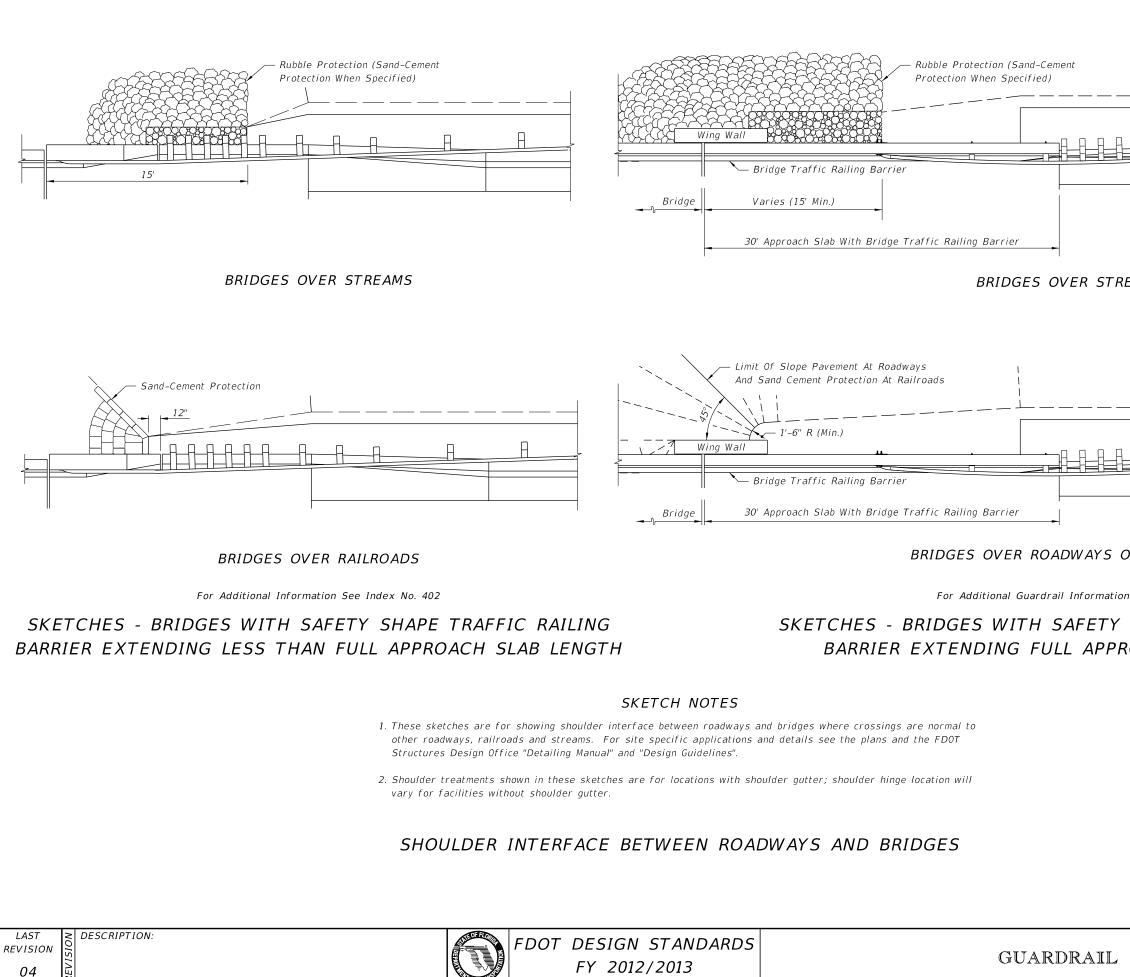


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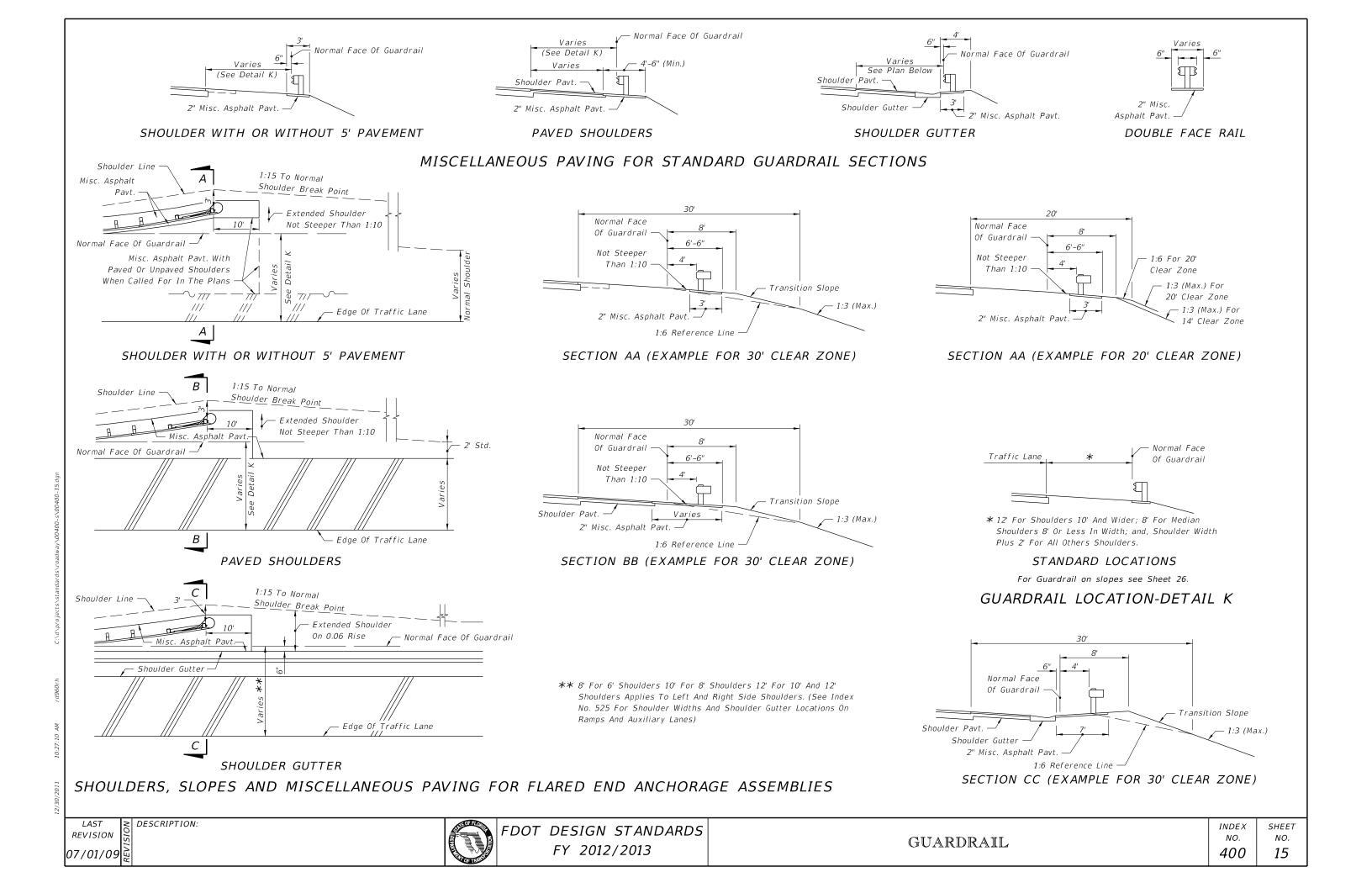


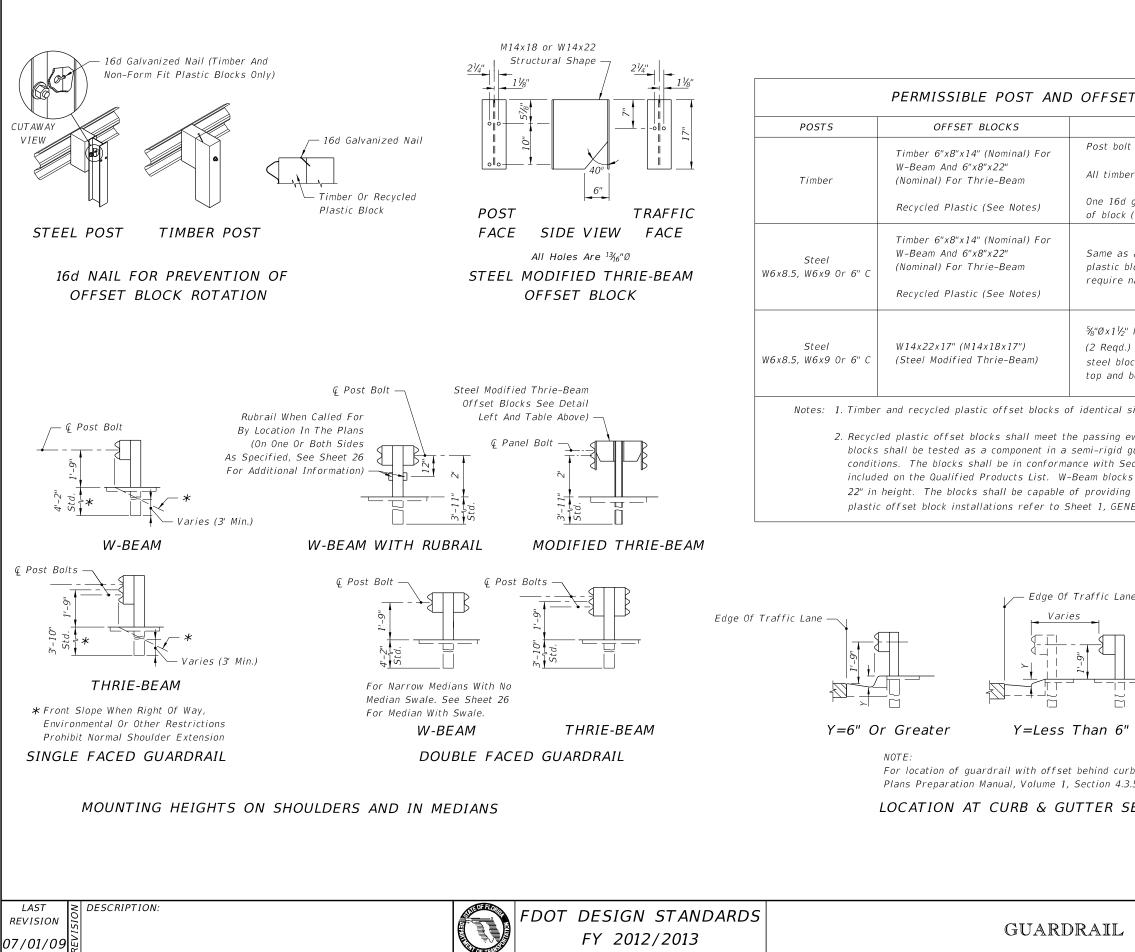
FY 2012/2013



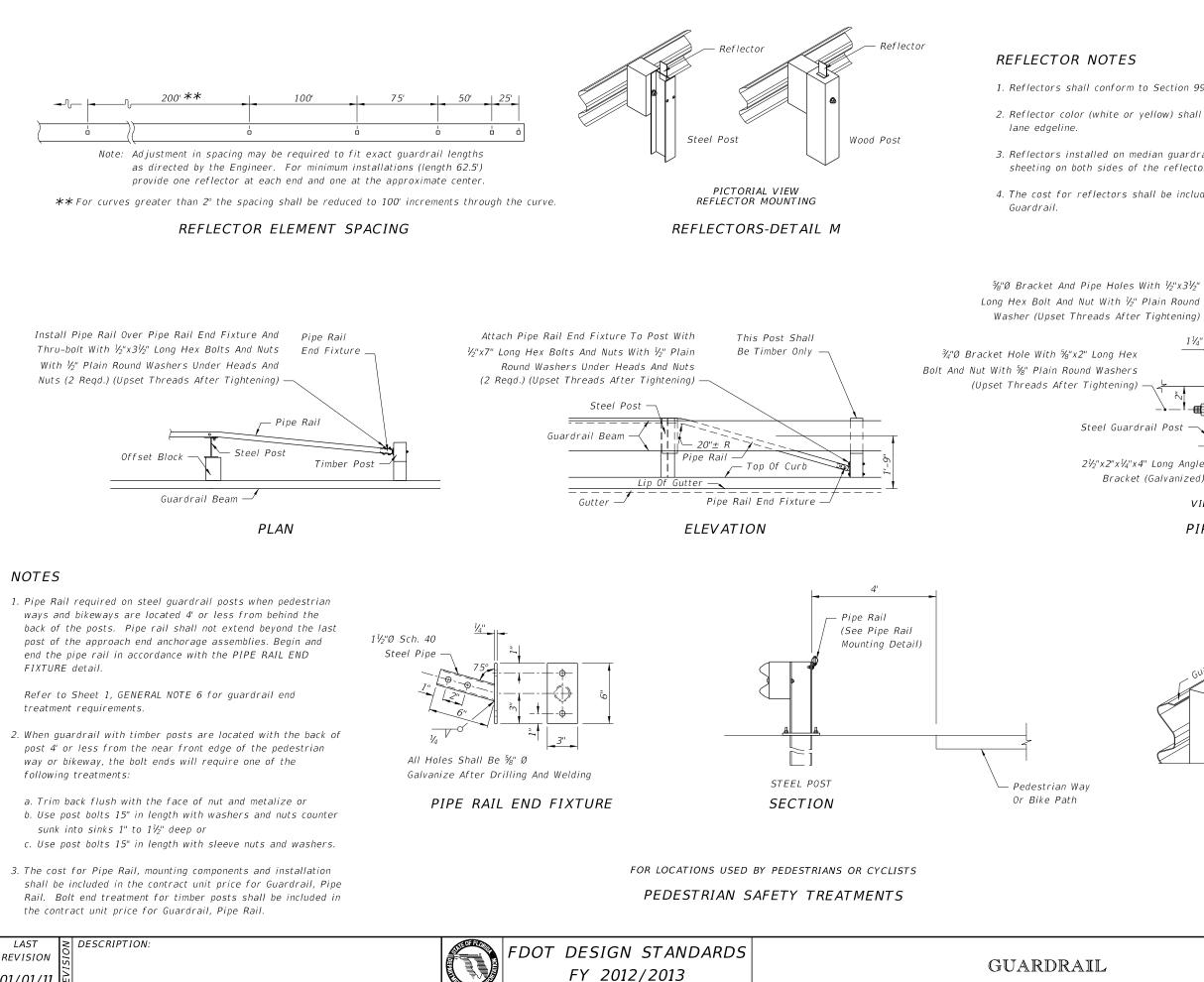


REAMS		
OR RAILROADS on See Sheet 13		
SHAPE TRAFFIC RAILING ROACH SLAB LENGTH	9	
	INDEX NO.	SHEET NO.
	400	14



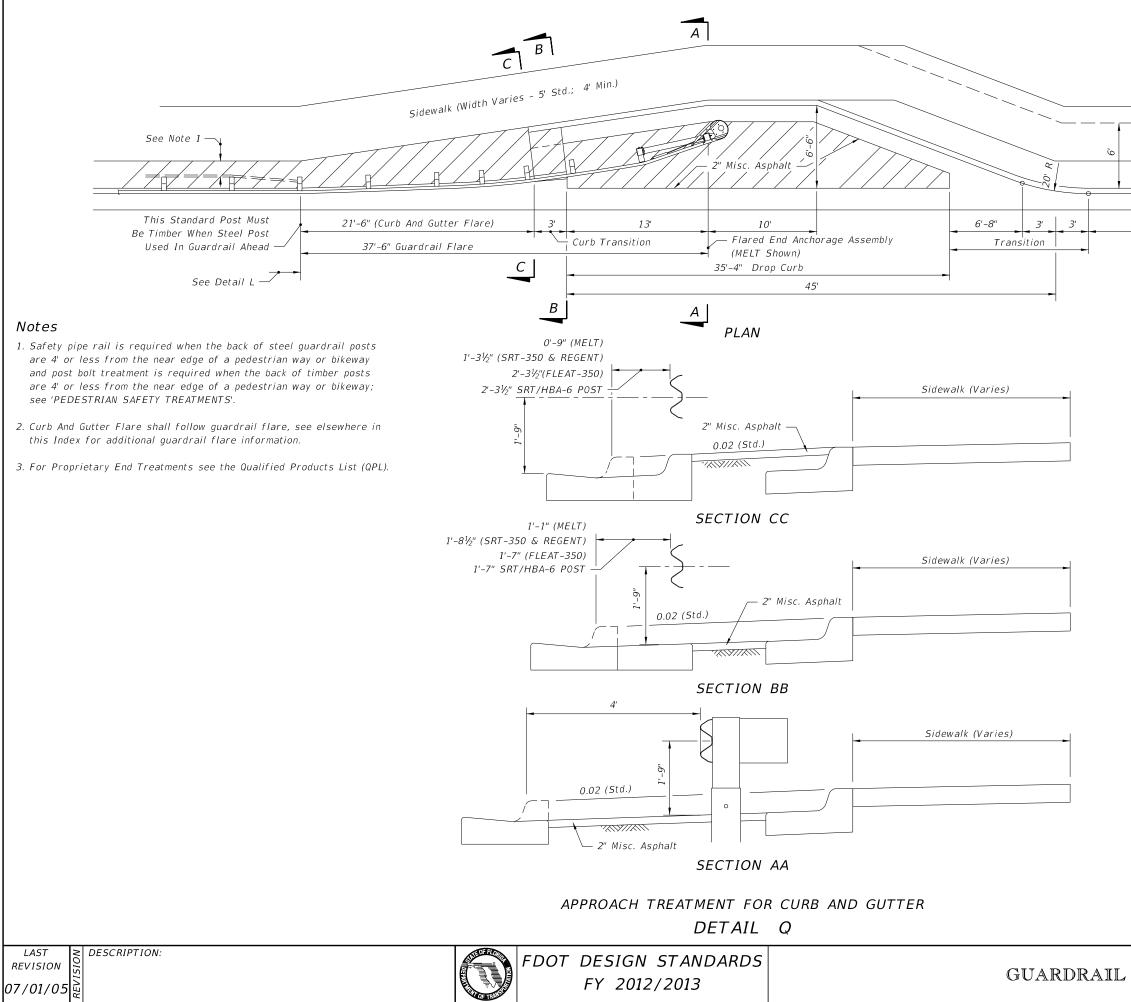


ЕТ ВLОСК СО	MBINATIONS		
	REMARKS		
olt hole in timber an	d plastic blocks to be cente	ered $\pm \frac{1}{4}$ ").	
ber offset blocks sh	all be dressed on all four	sides (S4S)	
od galvanized nail pe ck (see detail left).	r block is to be used to pro	event rotati	ion
	and plastic blocks except tl ith holes in steel posts and		t
d.) and ¾" plain rour	ts with full length thread a d washers (4 Reqd.) for mo are to be installed in oppos.	ounting	
I size and shape car	n be intermixed within a rui	n of rail.	
d guardrail test arti Sections 536 and 97 cks shall be 14" in h	for Test Level 3 crash test cle under full scale crash t 2 of the Specifications and eight and thrie-beam blocks et. For additional informati	est be s shall be	
ane		Should Line	ler
		index no. <b>400</b>	sheet NO. <b>16</b>

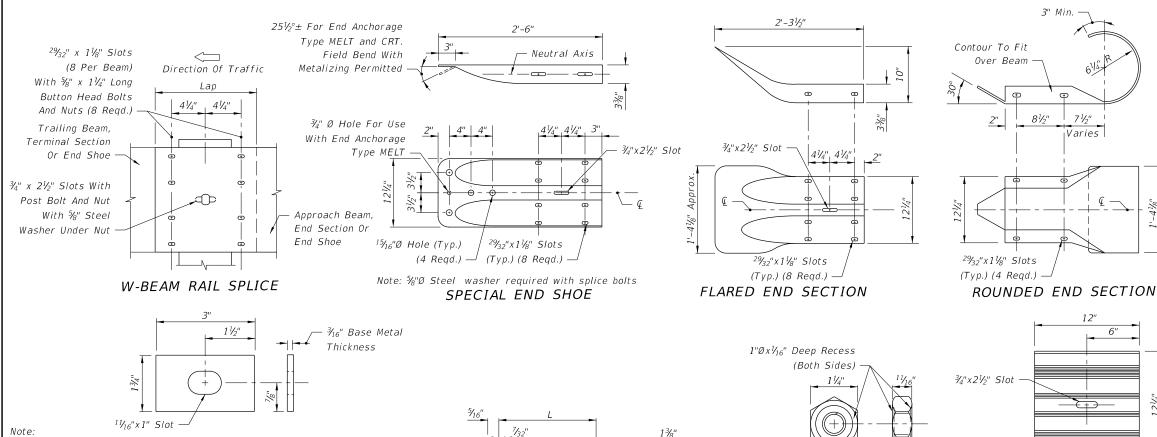


01/01/11

# 1. Reflectors shall conform to Section 993 of the Standard Specifications. 2. Reflector color (white or yellow) shall conform to the color of the near 3. Reflectors installed on median guardrail shall have retro-reflective sheeting on both sides of the reflector. 4. The cost for reflectors shall be included in the contract unit price for NPS 2 Sch. 40 Galv. Pipe Rail Per ASTM F1083 11/4" $(\oplus)$ - Steel Guardrail Post Steel Guardrail Post -\_1 ¼" + 11/4" 2<sup>1</sup>/<sub>2</sub>"x2"x<sup>1</sup>/<sub>4</sub>"x4" Long Angle 1½" Offset From 🤅 Bracket (Galvanized) Of Guardrail Post 4" VIEW A VIEW B PIPE RAIL MOUNTING offset Block VIEW R PICTORIAL SHEET INDEX NO. NO. 400 17

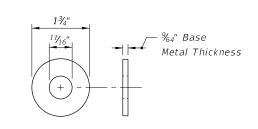


	/	
Sidewalk Without Utility Strip		
. /		
/		
10' Curb And Gutter Type	e F	
$\langle$		
	INDEX NO.	SHEET NO.
4	400	18



For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary to accomplish alignment or where the posts bolt head shows tendency to pull through the rail slot. Washers installed on guardrail, between end anchorages, prior to July 1, 1990 may remain in place until the guardrail is relocated or until repairs require removal and reinstallment of a post bolt.

# (RECTANGULAR PLATE WASHER) BEAM WASHER



Note:

The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by  $\frac{5}{8}$ " Ø hex bolts and nuts and under hex nut for connecting rubrail to wood and steel posts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

5/8" STEEL WASHER

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L (In.)	THREAD LENGTH (Min.) (In.)	APPLICATION	
1 1⁄4"	Full Length	Rail Splice Bolt	
10"	4"	Single Or Double Faced Guardrail Timber Or Recycled Plastic Offset Post Bolt - Block(s) On Steel Post As An Option, A Single 25"* Long Post Bolt May Be Used	
18"	4"	Post Bolt – Single Faced Guardrail Timber Posts	
25"*	4"	Post Bolt - Double Faced Guardrail Timber Posts Double Faced Guardrail Steel Posts	

Special bolts having lengths of 10" or greater shall have a thread length of not less than 4".

For applications where special bolts having lengths greater than 25" are required, the Contractor may use a 5/8"Ø threaded rod (field cut to length). A hex nut and beam washer shall be used at the guardrail face with no more than  $\frac{3}{4}$ " of the threaded rod projecting beyond the top of the nut. The projecting thread on both ends shall be distorted to secure the nuts, and both ends of the threaded rod metalized with organic zinc-rich coating.

\*Use of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced guardrail that results in the bolt projecting more than  $\frac{3}{4}$  beyond the face of the nut after pull-up shall be trimmed to  $\frac{3}{4}$ " reveal and metalized with organic zinc-rich coating.

5/8" OVAL SHOULDER BUTTON HEAD BOLT

Note: For application information see individual

OFFSETS (Ft.)						
Me	Measured From Face Of Guardrail To					
Front Of Above Ground Rigid Hazard						
POST	SINGLE BEAM		NESTE	D		
SPACING (Ft.)	W-Beam	Thrie-Beam	W-Beam	7		
6'-3"	4'-0"	3'-4''	N/A	┢		
3'-11/2"	3'-0"	2'-8''	2'-8"			
1'-6¾"	N/A	N/A	2'-4"			
Note						

The values shown should be utilized unless changes are supported by empirical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5.4 of the AASHTO Roadside Design Guide are cautioned to proceed only if background in the table development is understood.

# MINIMUM OFFSET FOR SINGLE FACED GUARDRAIL (Ft.)

HS Hex bolts for THRIE-BEAM TERMINAL CONNECTORS shall conform to the requirements of ASTM A449 (Type 1) with heavy hex nuts and washers. All other hex bolts shall conform to the requirements of ASTM A563. Bolts, nuts and washers shall be hot dip galvanized. Heavy hex nut may be used in lieu of hex nuts and hex nuts used for jam nuts.

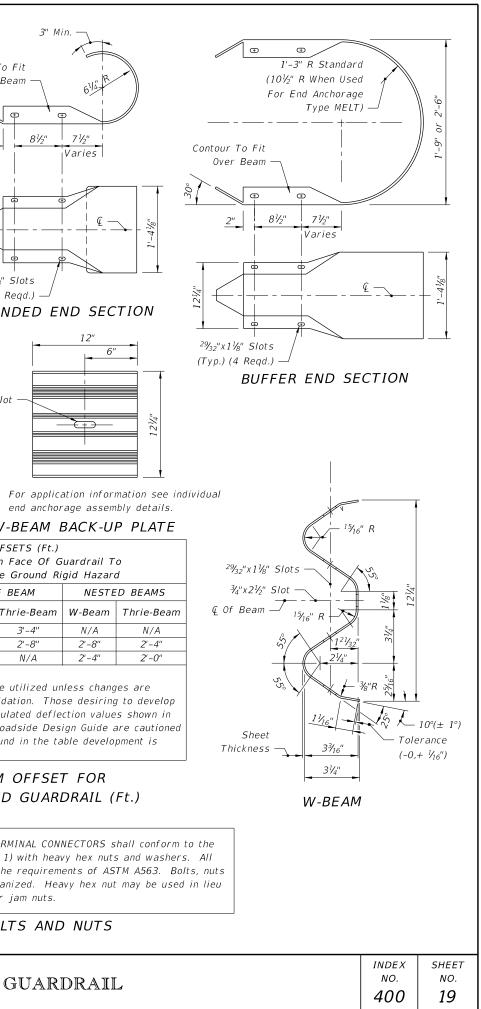
# HEX BOLTS AND NUTS

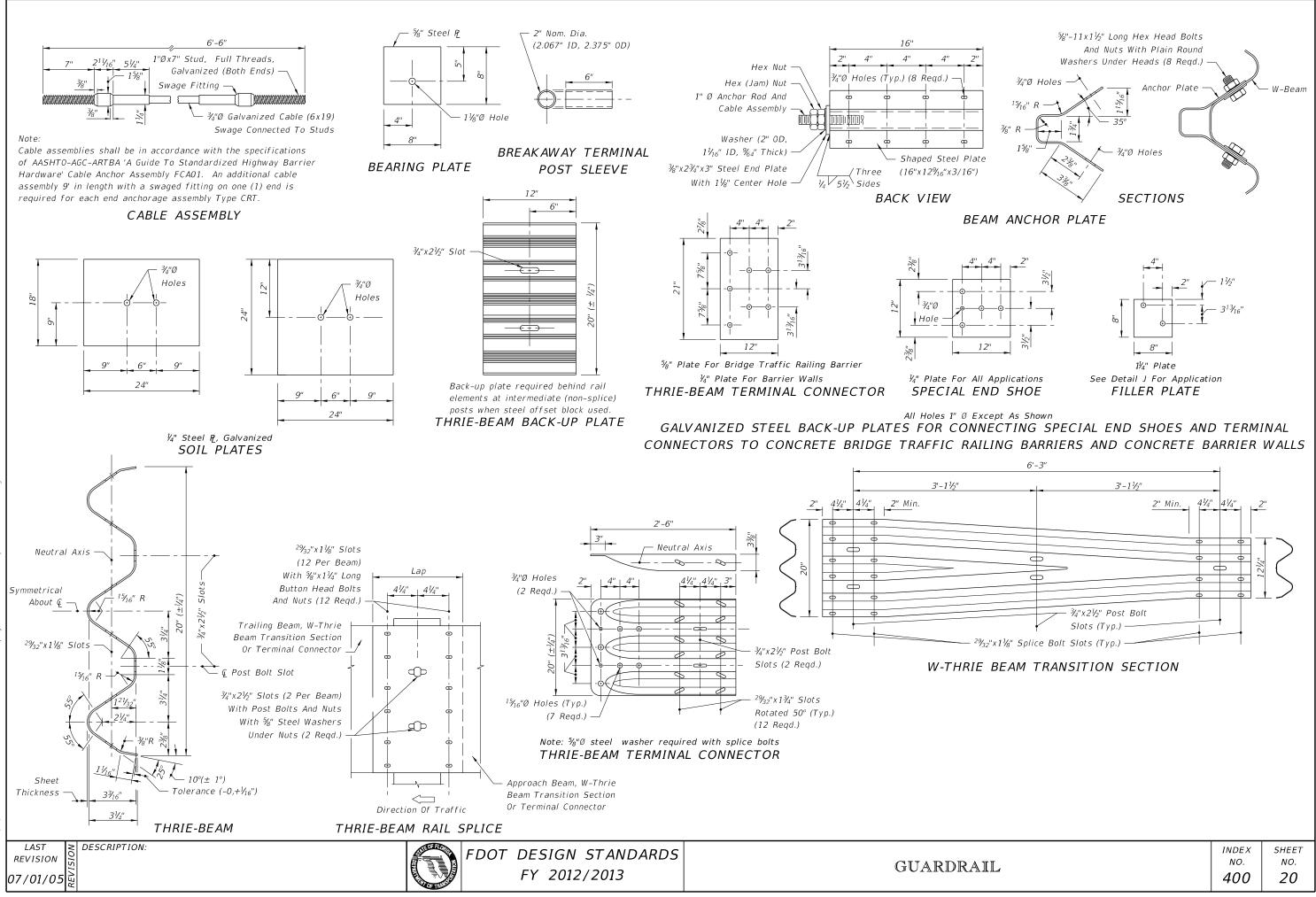


5/8" MODIFIED HEAVY

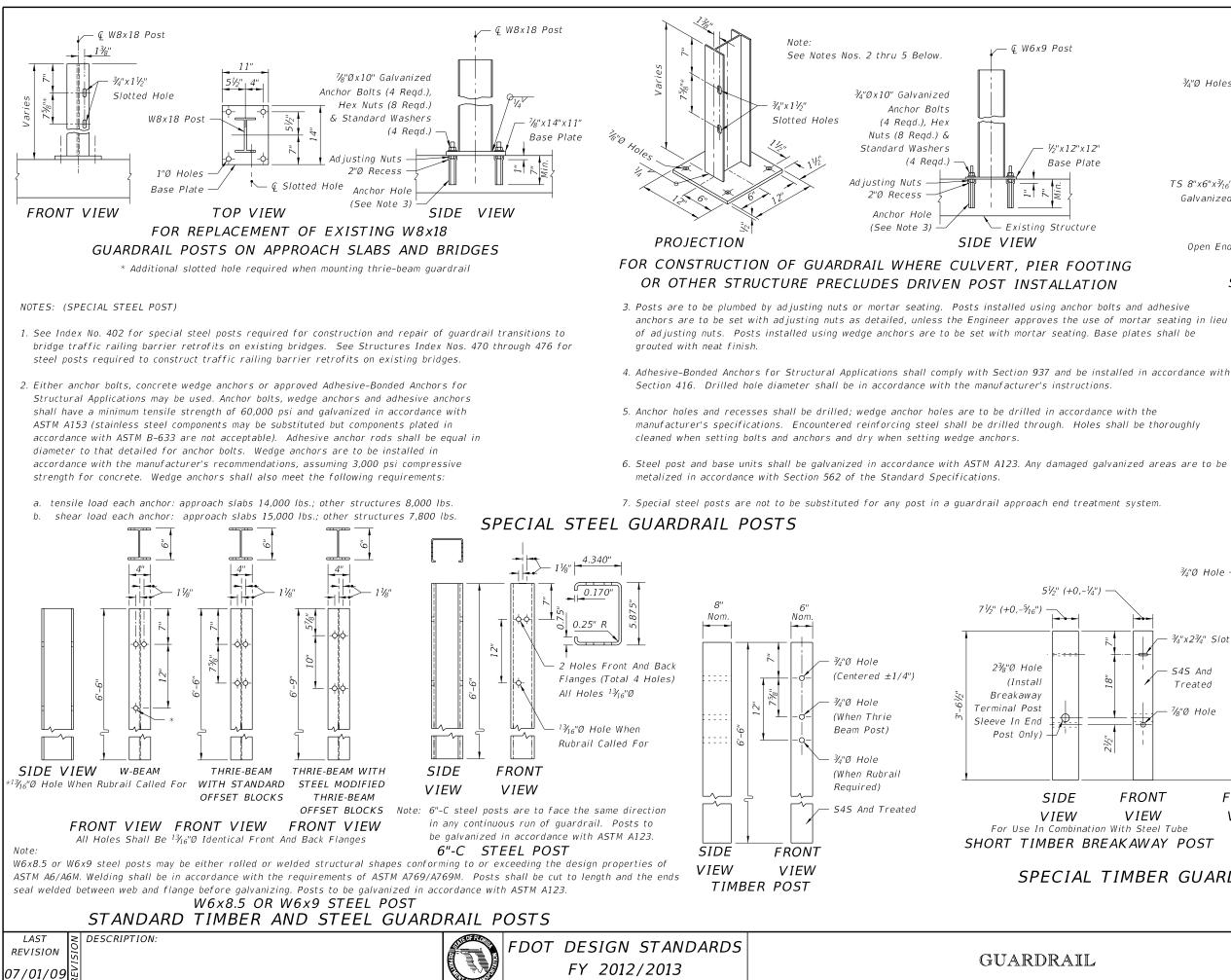
HEX NUT (RECESSED NUT)

end anchorage assembly details. W-BEAM BACK-UP PLATE





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GUARDRAIL

 $\rightarrow$ 

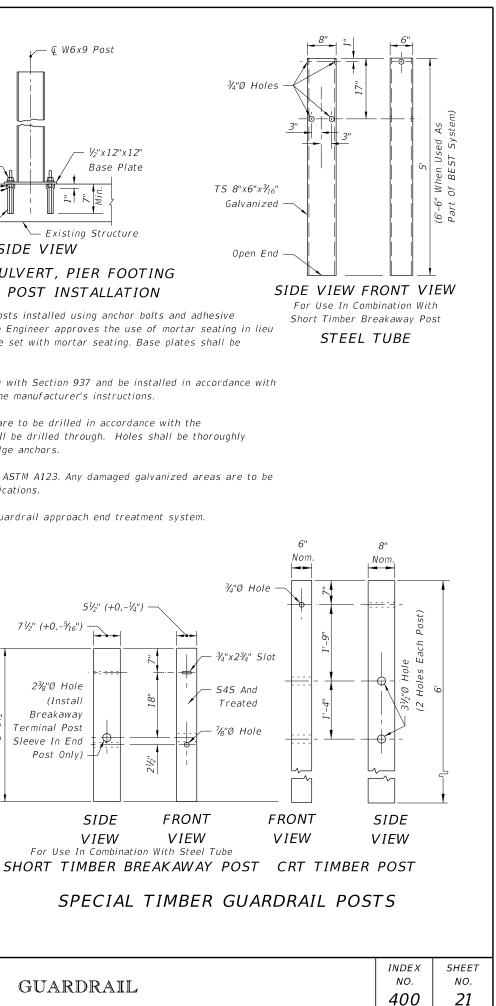
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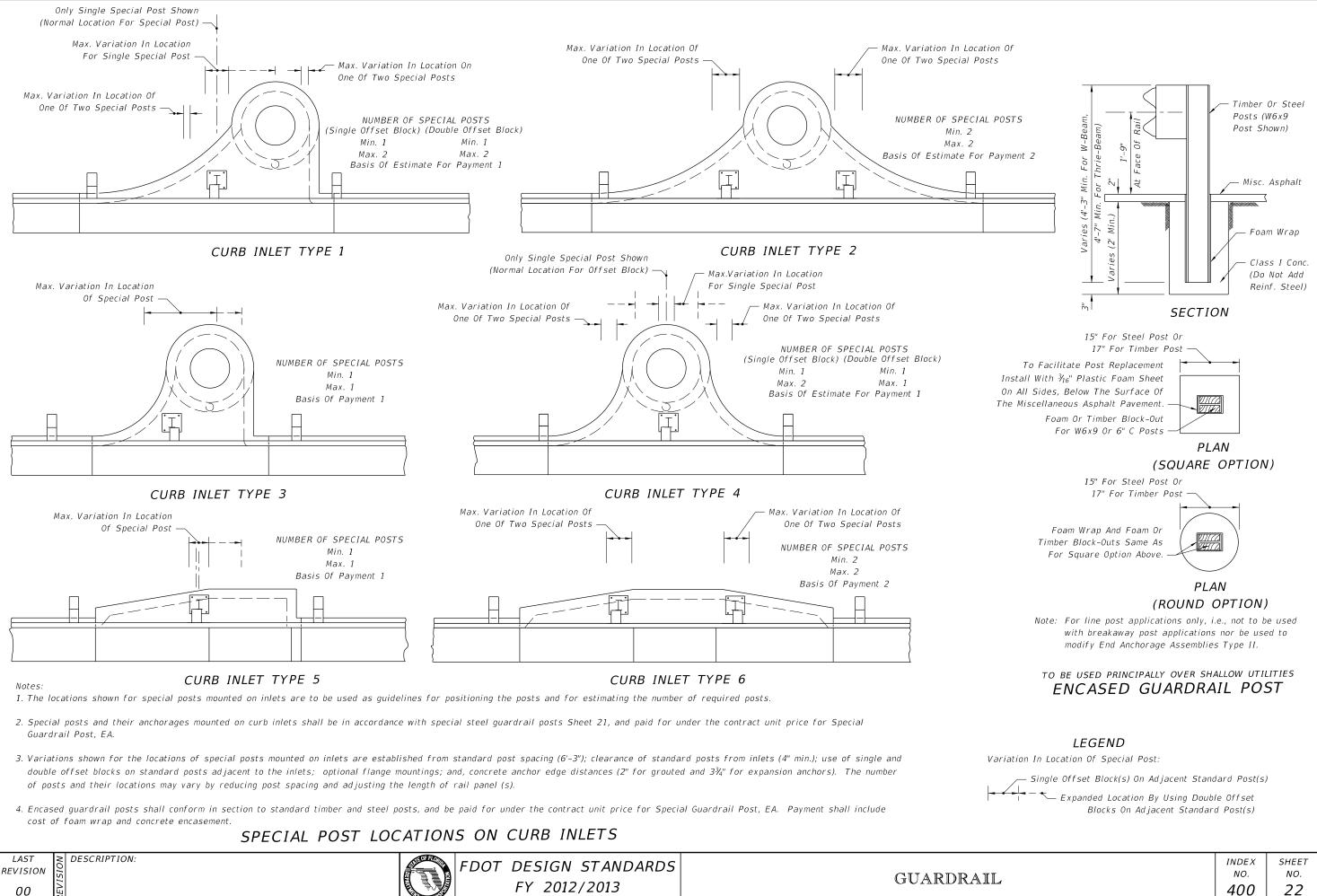
SIDE

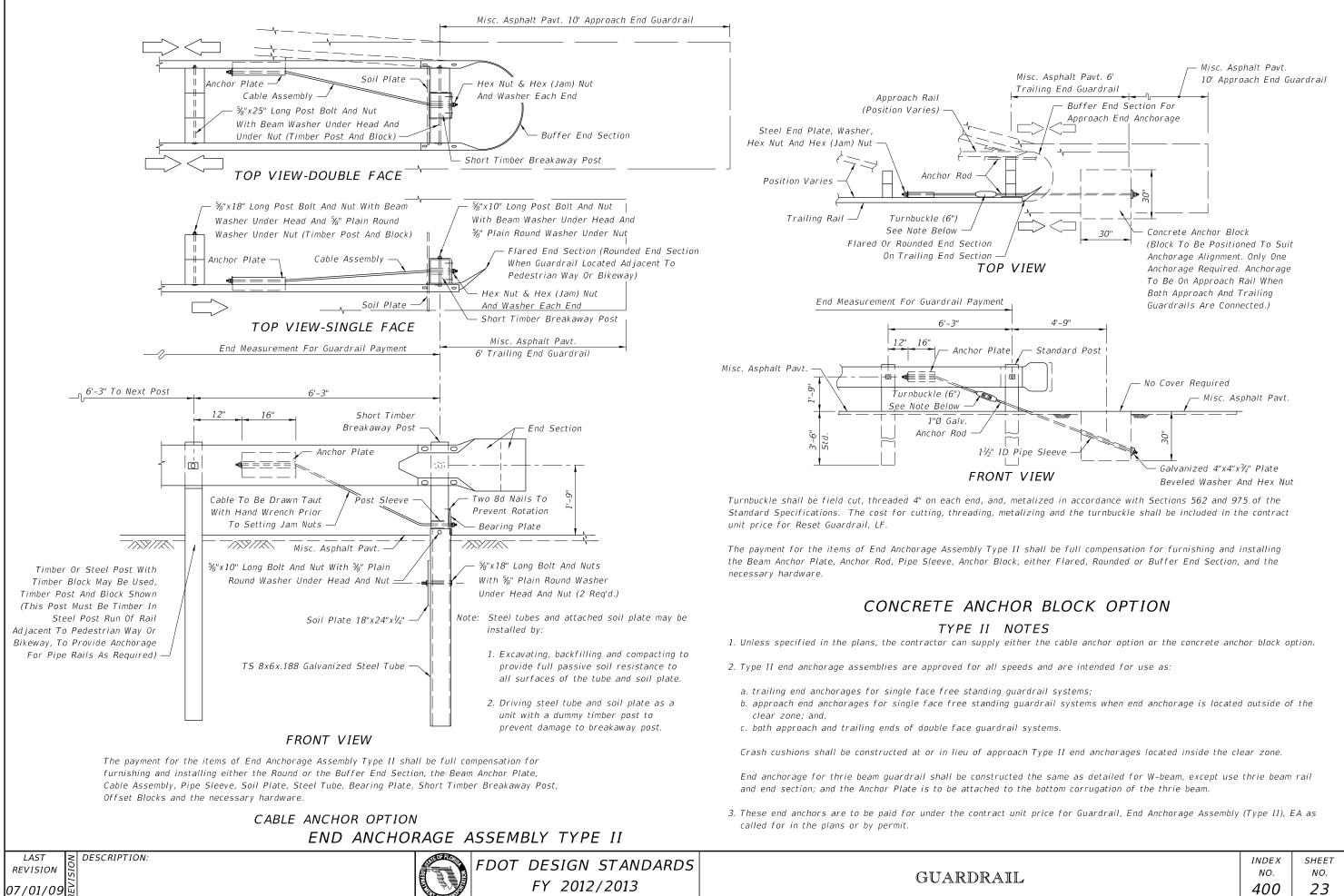
VIEW

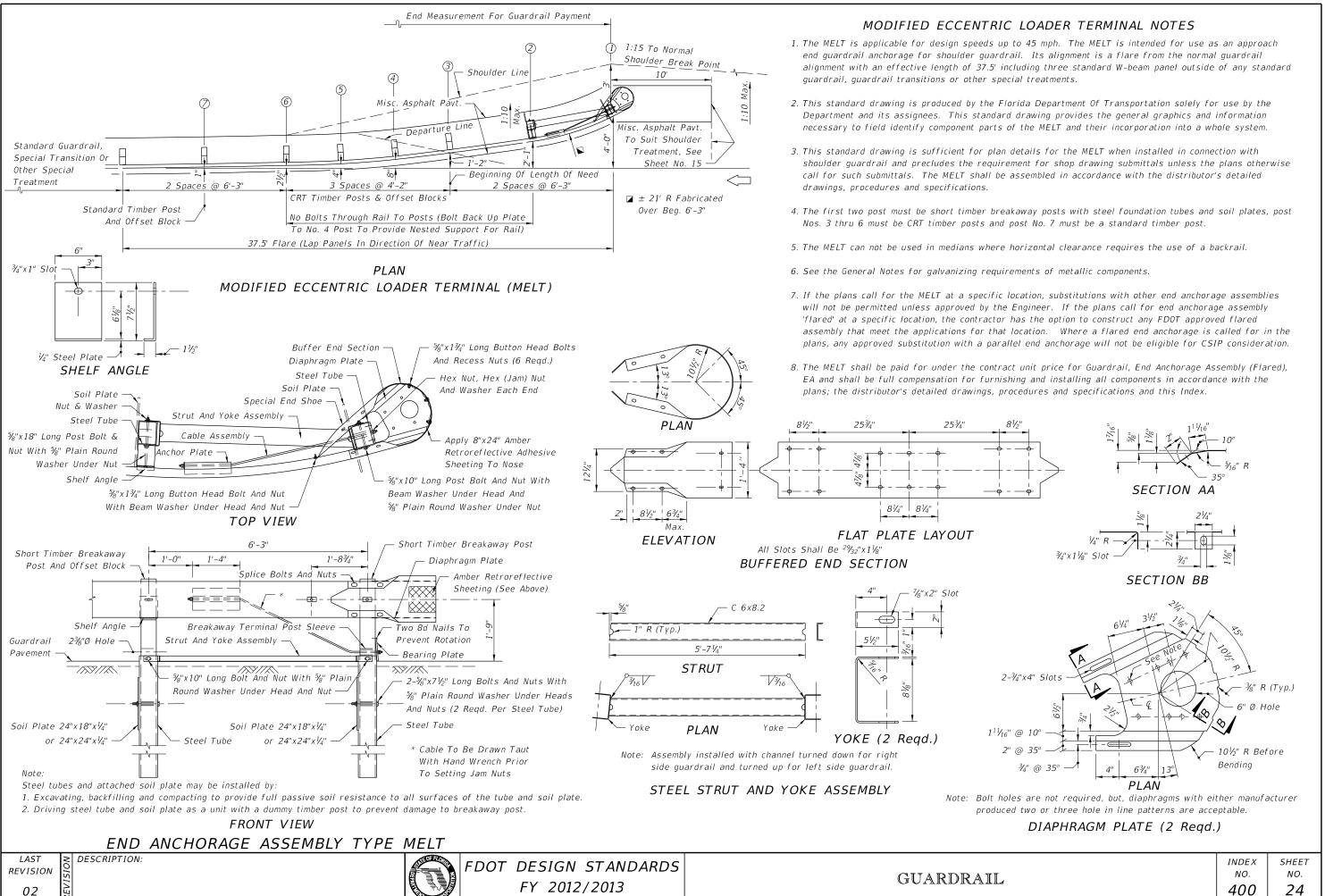
(Install

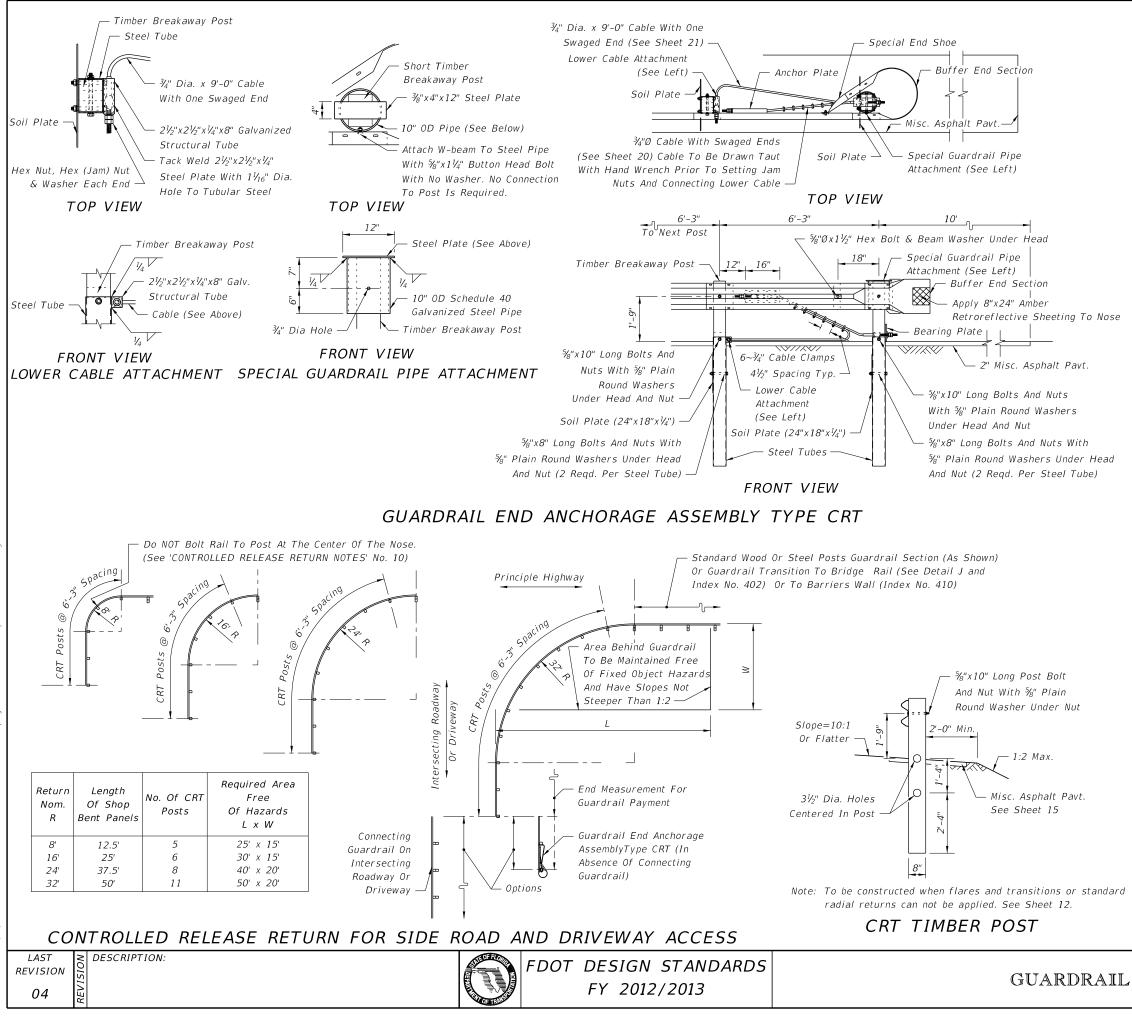
@ W6x9 Post







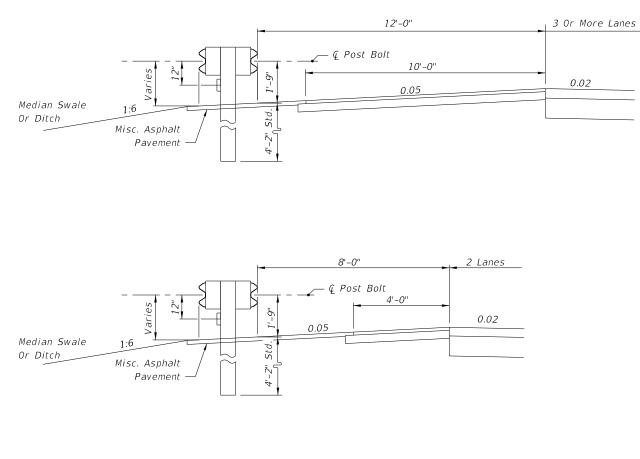




# CONTROLLED RELEASE RETURN NOTES

- Controlled release returns are intended for use (a) in openings in continuous guardrail for driveway and side road access when flares and transitions or standard radial returns can not be applied (Sheet 12); and, (b) for shielding the ends of bridge traffic rails and barrier walls where the driveway and side road access is in close proximity to the structure and space does not permit the proper use of approved flared and parallel types of Guardrail End Anchorage Assemblies.
- 2. Controlled release returns are not intended as a substitute or replacement for the appropriate use of approved vehicle impact attenuators.
- 3. Controlled release returns with either 8', 16' or 24' radii are designed for highway speeds of 60 mph or less.
- 4. The controlled release returns shown are designed as full returns based on an intersection angle of 90°. The return can be terminated with the Guardrail End Anchorage Assembly Type CRT or connected to standard guardrail as shown or as otherwise detailed in the plans.
- 5. The Guardrail End Anchorage Assembly Type CRT is to be used only for the controlled release returns with 8', 16', 24' and 32' radii as shown; the assembly is not to be used in any tangent rail or flared rail applications. Other types of end anchorage assemblies are not to be used in the controlled release returns.
- 6. The area immediately behind the control release return shall have slopes not steeper than 1:2 and be maintained free of fixed objects in accordance with the area limits tabulated in the plan below.
- 7. The surface approaching the controlled release return shall have a transverse slope not exceeding 1:10. The effective width of the transverse surface is to be based on standard vehicle departure, return radii and preceding shielding; the width (beyond shoulder) shall be not greater than the corresponding 15' and 20', 'W' values tabulated below.
- 8. The curved guardrail portion of the controlled release return shall be full section shop bent panels (12.5' or 25' panels).
- 9. Washers are not to be used between the guardrail beam and the head of the button head post bolts at any controlled release terminal (CRT) post or at any Guardrail End Anchorage Assembly Type CRT breakaway timber post.
- 10. The guardrail beam of the 8' radius return is not bolted to the center control release post.
- 11. See the General Notes for galvanizing requirements of metallic components.
- 12. Controlled release return systems shall be paid for under the contract unit prices for Guardrail (Roadway), LF, Guardrail (Shop-bent Panels), LF, and Guardrail, End Anchorage Assembly (Type CRT), EA as called for in the plans or by permit and shall be full compensation for furnishing and installing all components in accordance with the plans and with this index. CRT posts are included in the cost for guardrail.

INDEX	SHEET
NO.	NO.
400	25



Notes:

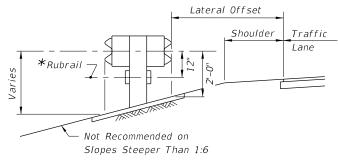
1. Typical placement shown. May be constructed at other locations as called for in the plans.

2. Rubrail required on median side or ditch side of barrier.

# MOUNTING HEIGHT FOR DOUBLE FACED GUARDRAIL ON MEDIAN SHOULDERS (FREEWAYS)

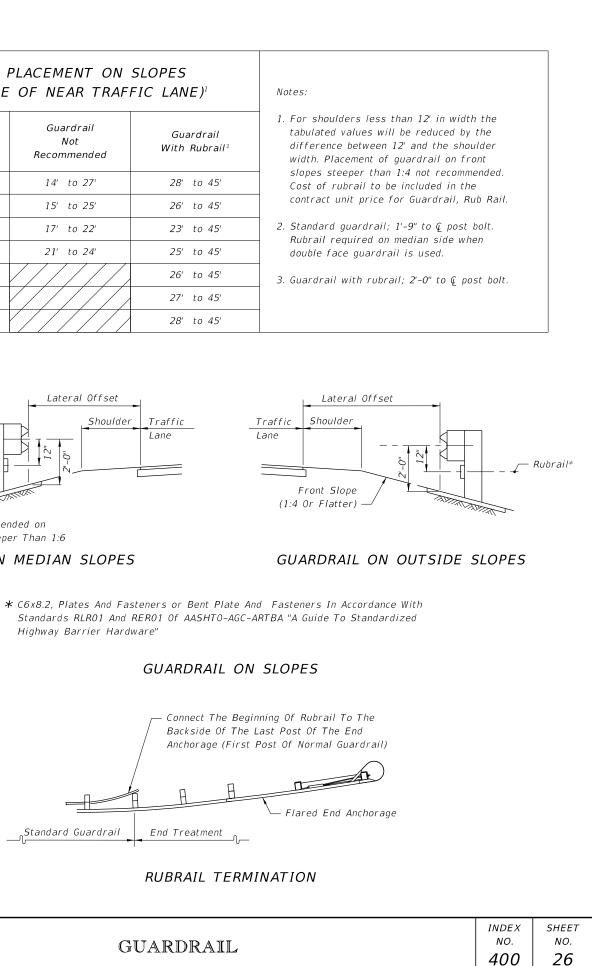
# LATERAL PLACEMENT ON SLOPES (FROM EDGE OF NEAR TRAFFIC LANE)<sup>1</sup>

Slope	Standard Guardrail²	Guardrail Not Recommended	Guardrail With Rubrail <sup>3</sup>
1:4	to 13'	14' to 27'	28' to 45'
1:5	to 14'	15' to 25'	26' to 45'
1:6	to 16'	17' to 22'	23' to 45'
1:7	to 20'	21' to 24'	25' to 45'
1:8	to 25'		26' to 45'
1:9	to 26'		27' to 45'
1:10	to 27'		28' to 45'

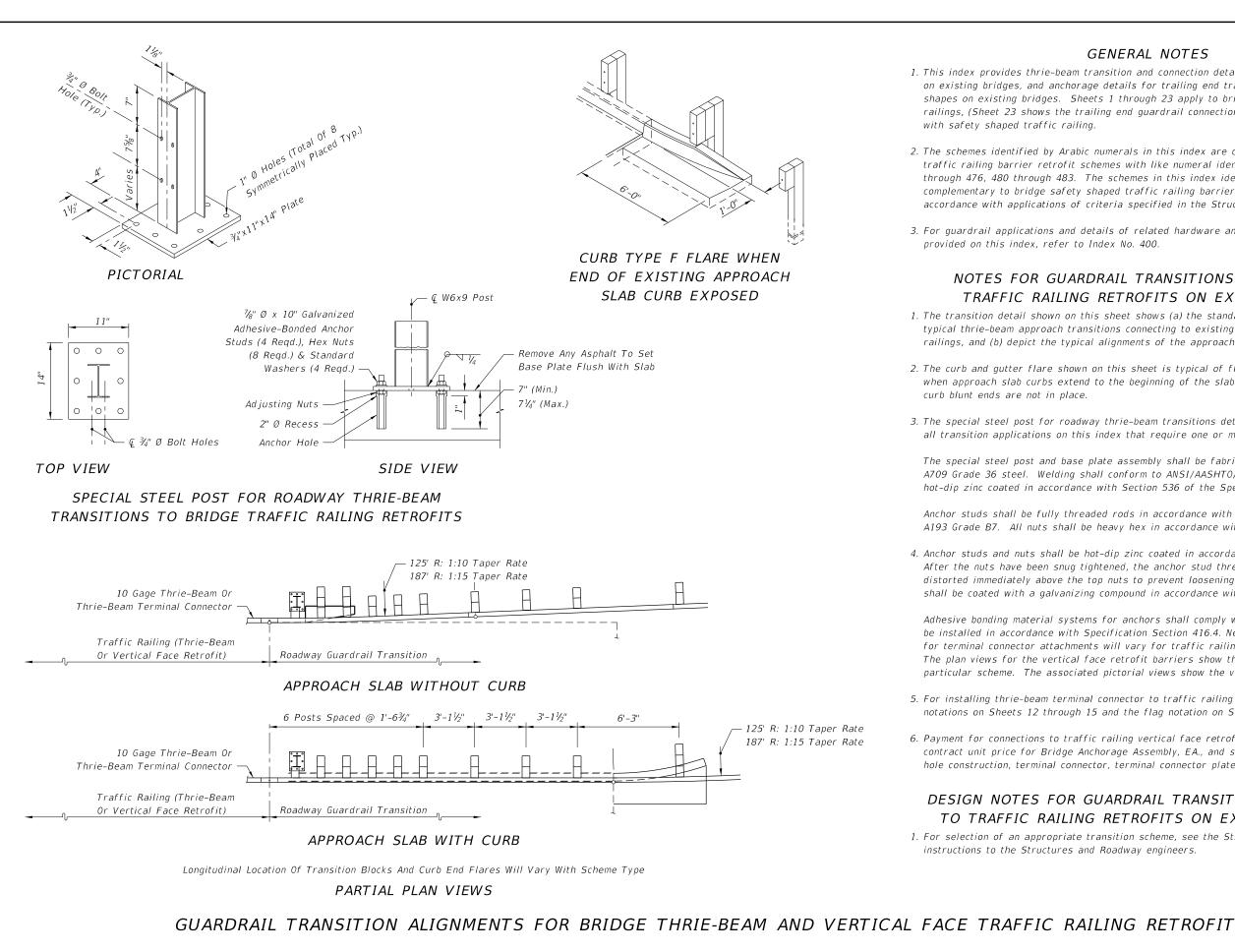


# GUARDRAIL ON MEDIAN SLOPES

Highway Barrier Hardware"



ALLEU TOOL
CETTER ST





FDOT DESIGN STANDARDS FY 2012/2013

INDEX SHEET GUARDRAIL TRANSITIONS AND NO. NO. CONNECTIONS FOR EXISTING BRIDGES 402 1

- with safety shaped traffic railing.
- provided on this index, refer to Index No. 400.

# NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- curb blunt ends are not in place.

# DESIGN NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

instructions to the Structures and Roadway engineers.

### GENERAL NOTES

1. This index provides thrie-beam transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 23 apply to bridges with retrofitted traffic railings, (Sheet 23 shows the trailing end guardrail connections). Sheet 24 applies to bridges

2. The schemes identified by Arabic numerals in this index are complementary to the bridge traffic railing barrier retrofit schemes with like numeral identification in Index Nos. 470, 471 through 476, 480 through 483. The schemes in this index identified by Roman numerals are complementary to bridge safety shaped traffic railing barrier where determined to be in accordance with applications of criteria specified in the Structures Manual.

3. For guardrail applications and details of related hardware and accessories that are not

1. The transition detail shown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.

2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to

3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated using ASTM A36 or ASTM A709 Grade 36 steel. Welding shall conform to ANSI/AASHTO/AWS D1.5. The assembly shall be hot-dip zinc coated in accordance with Section 536 of the Specifications.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A19

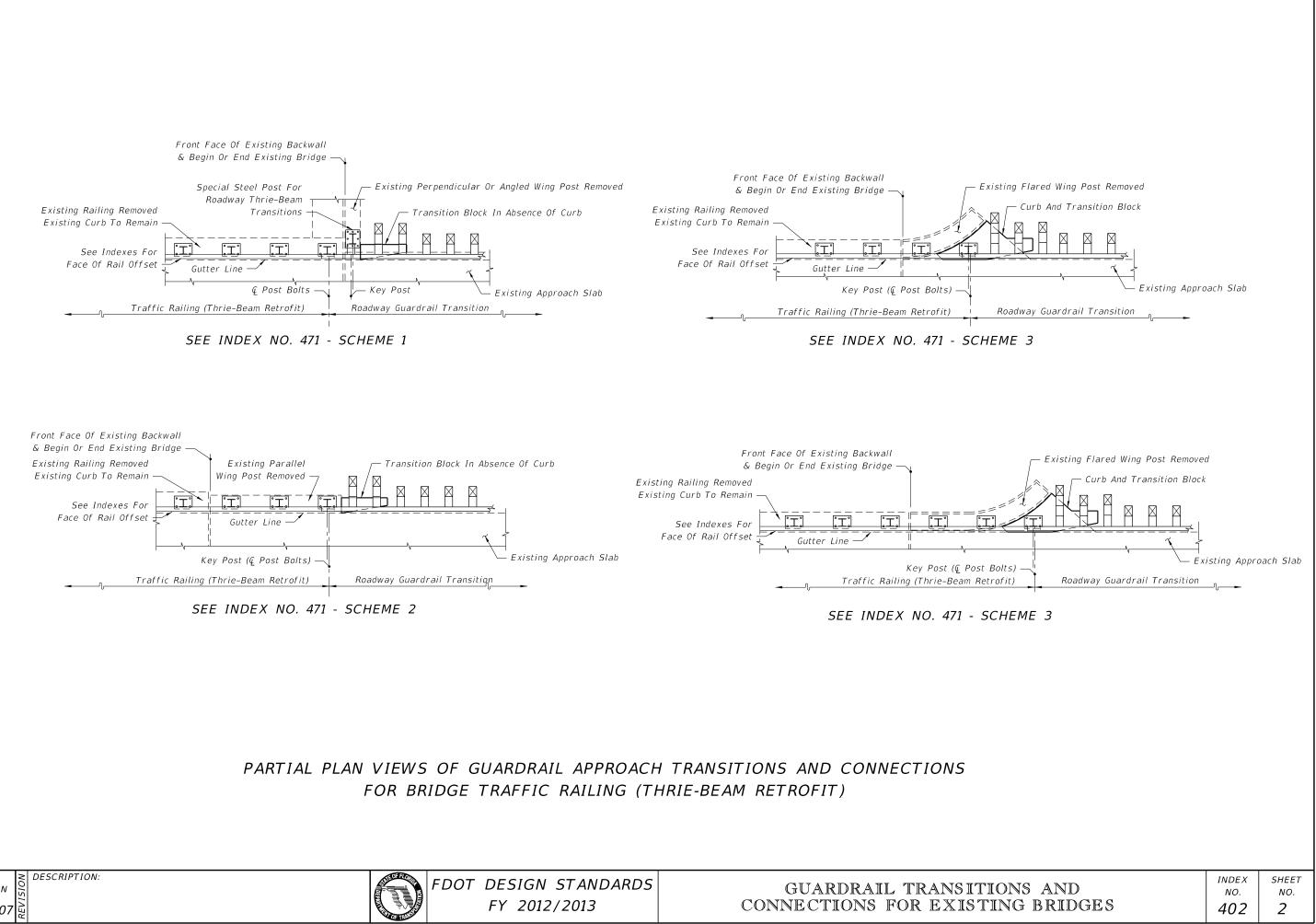
4. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

Adhesive bonding material systems for anchors shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 12 through 15 and the flag notation on Sheet 23.

6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

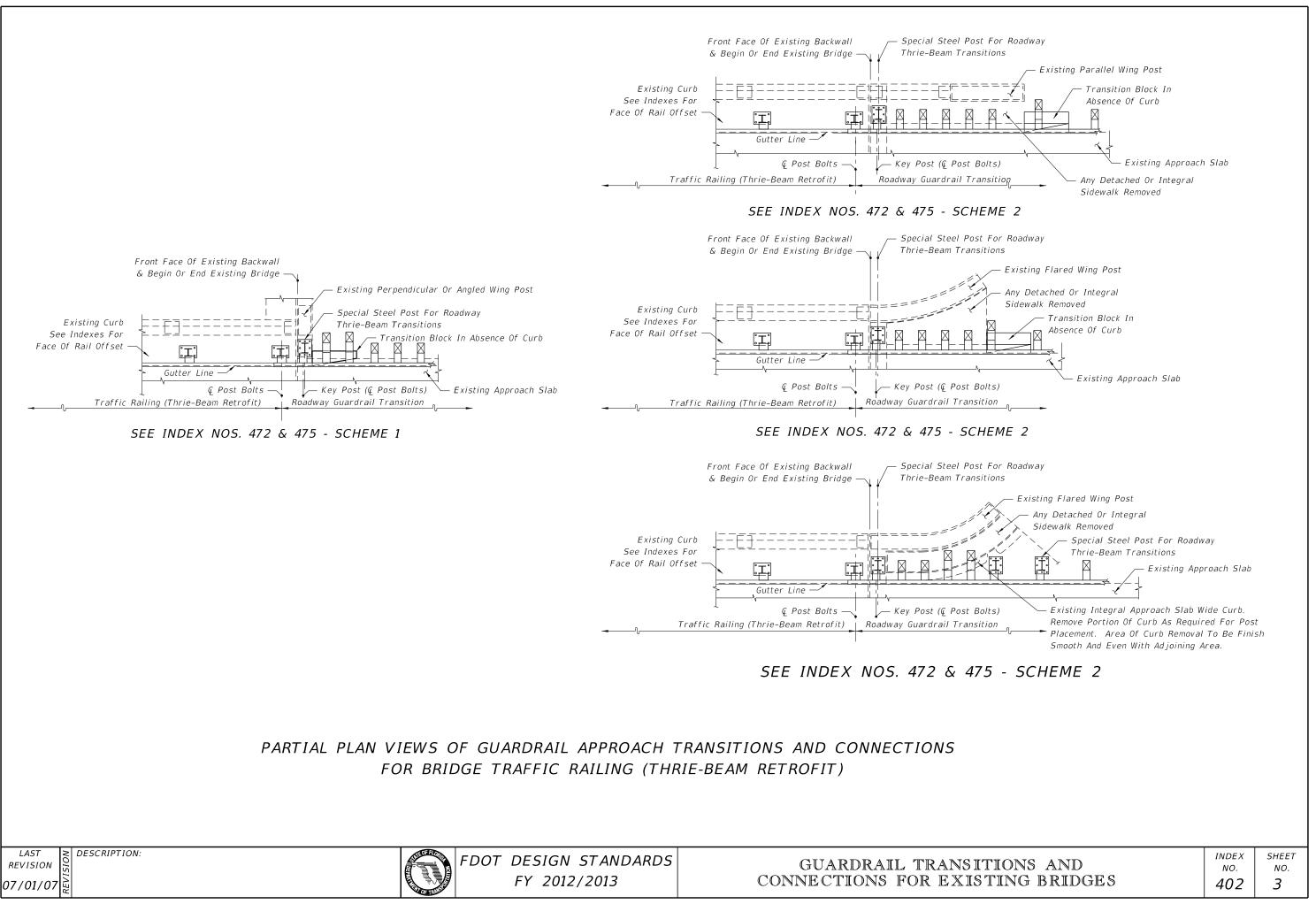
1. For selection of an appropriate transition scheme, see the Structures Manual for

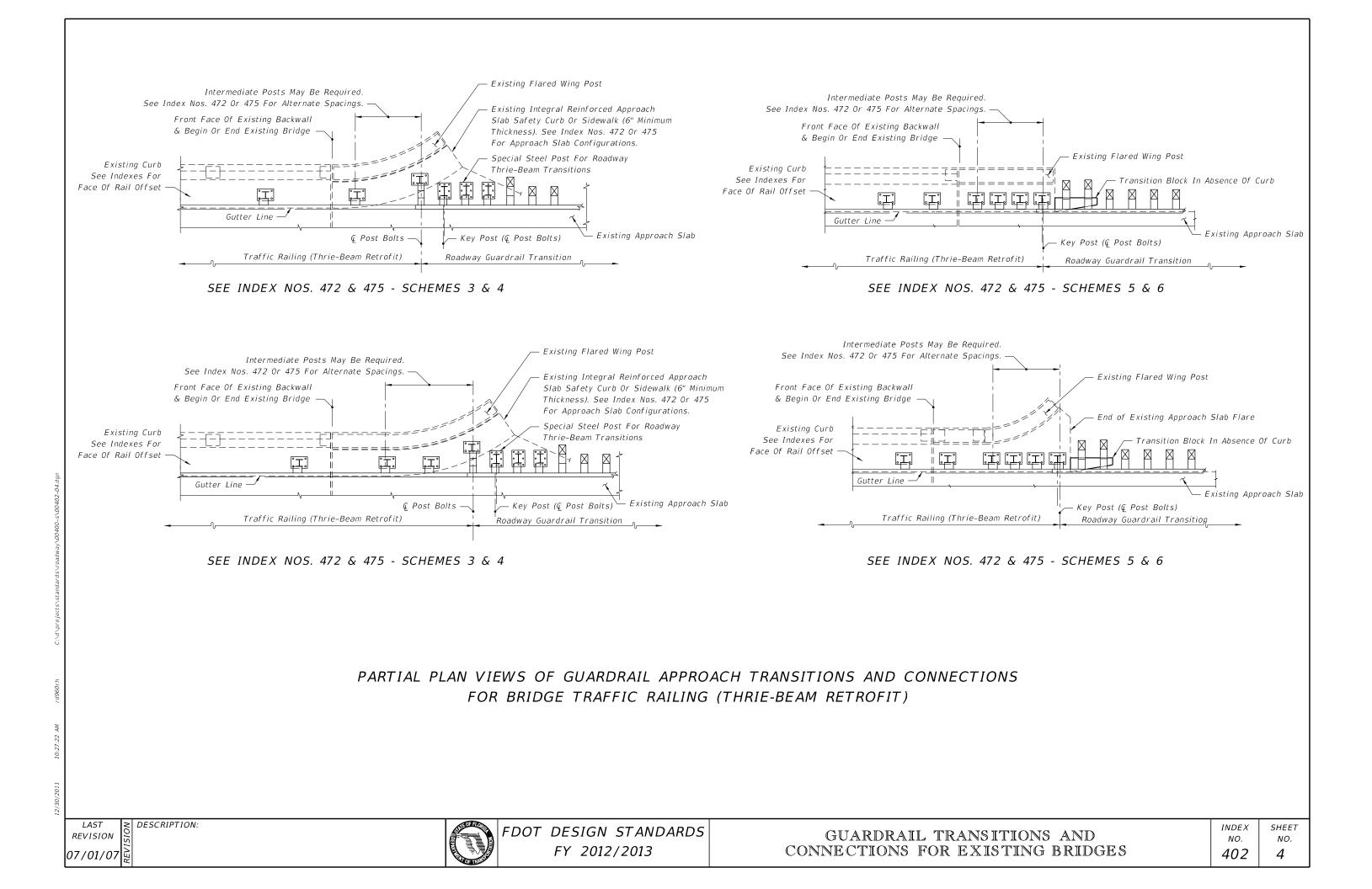


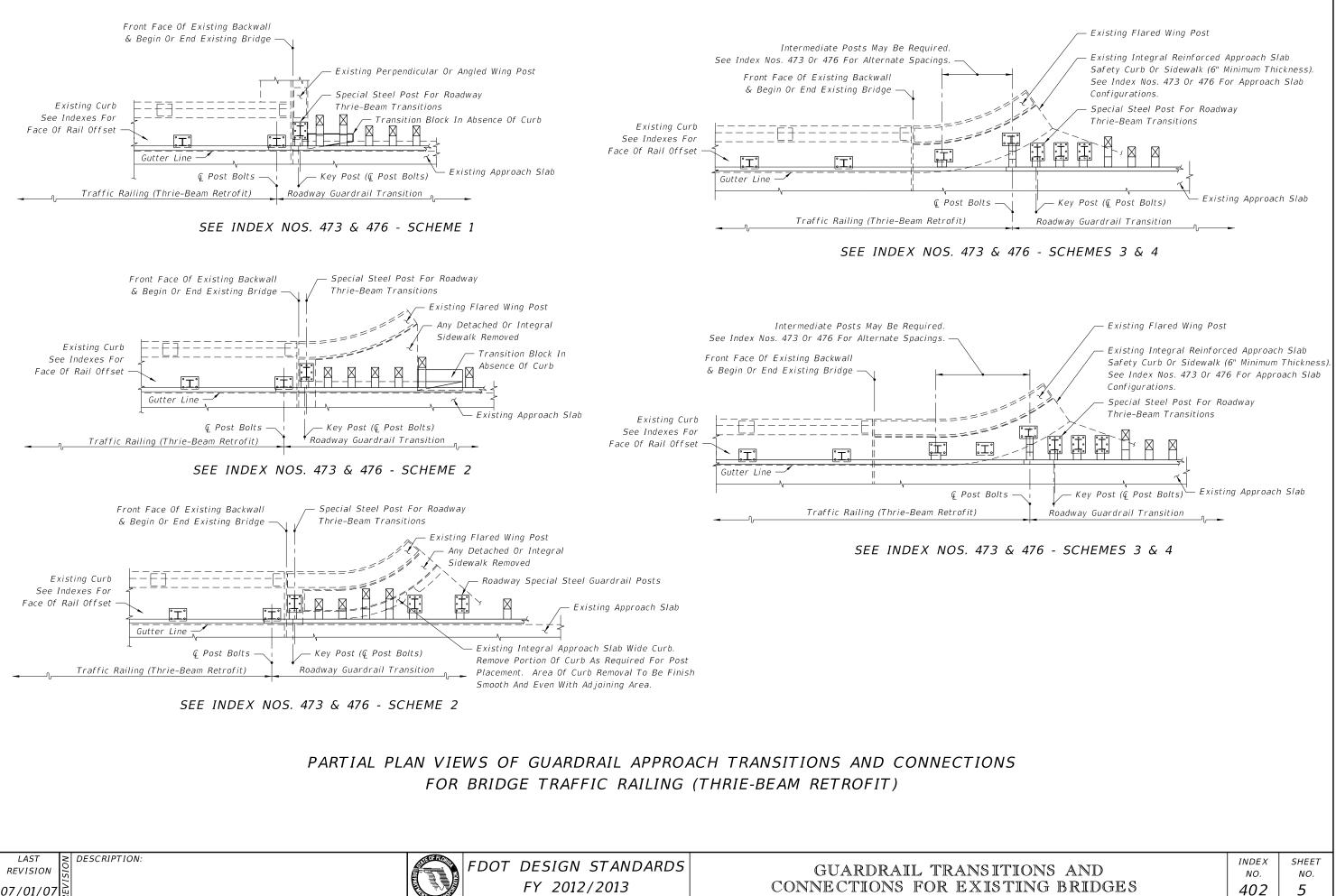
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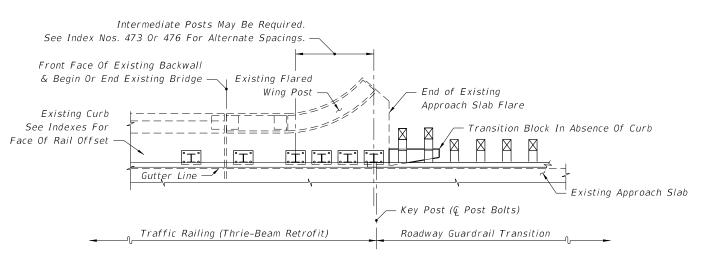




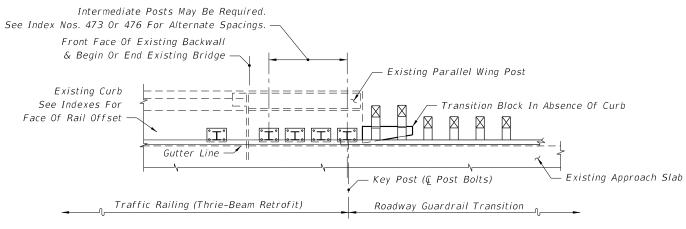


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SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

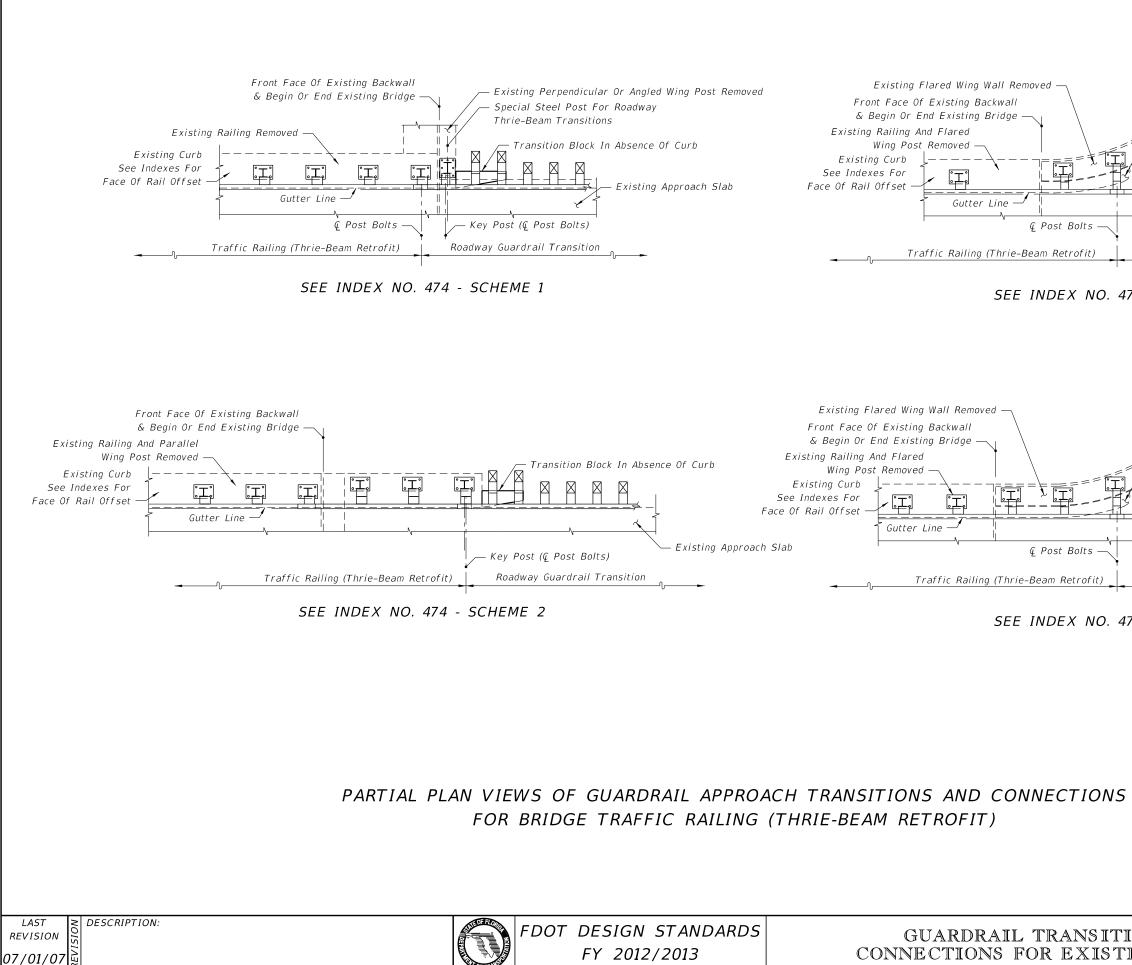


SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6

# PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

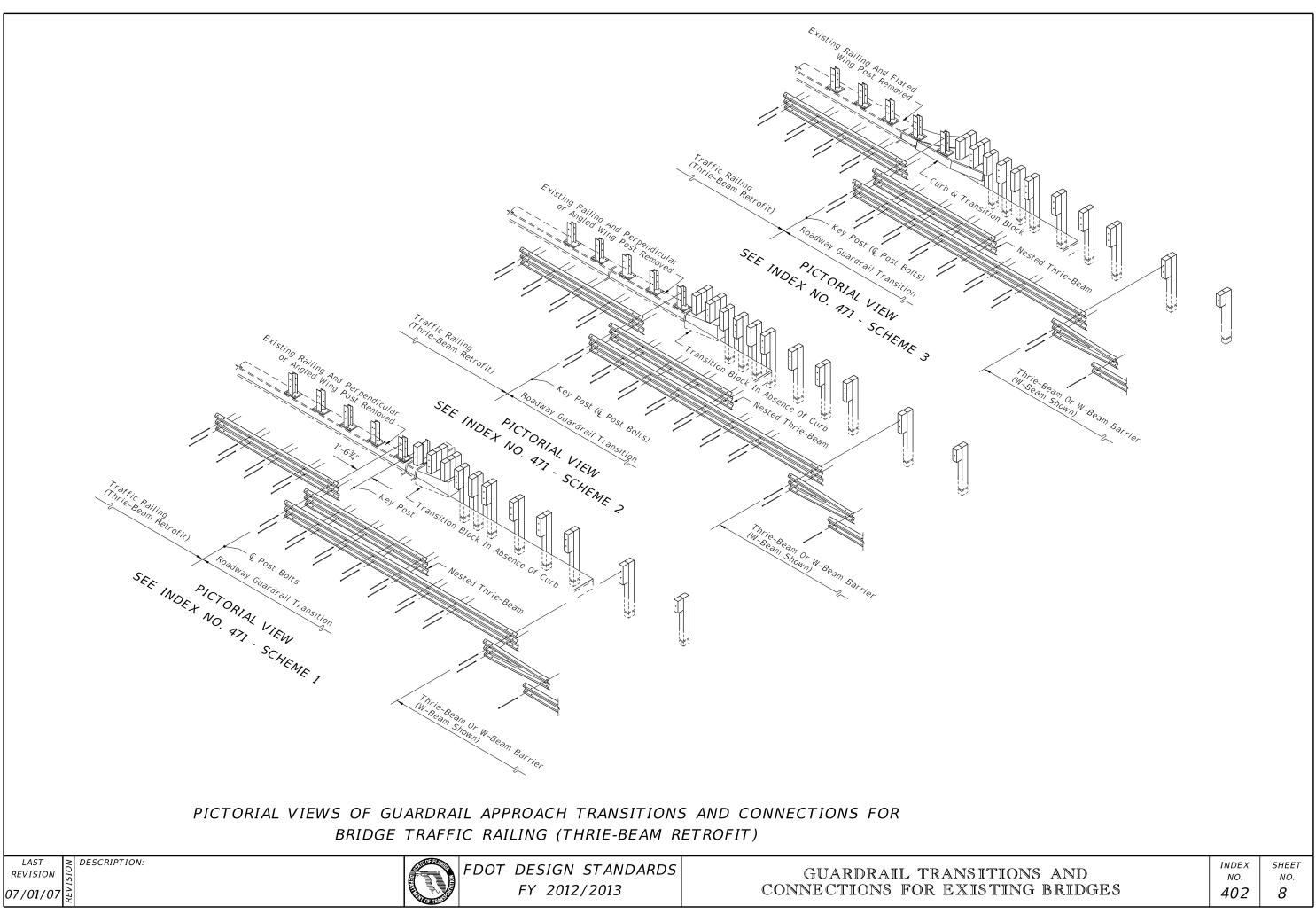
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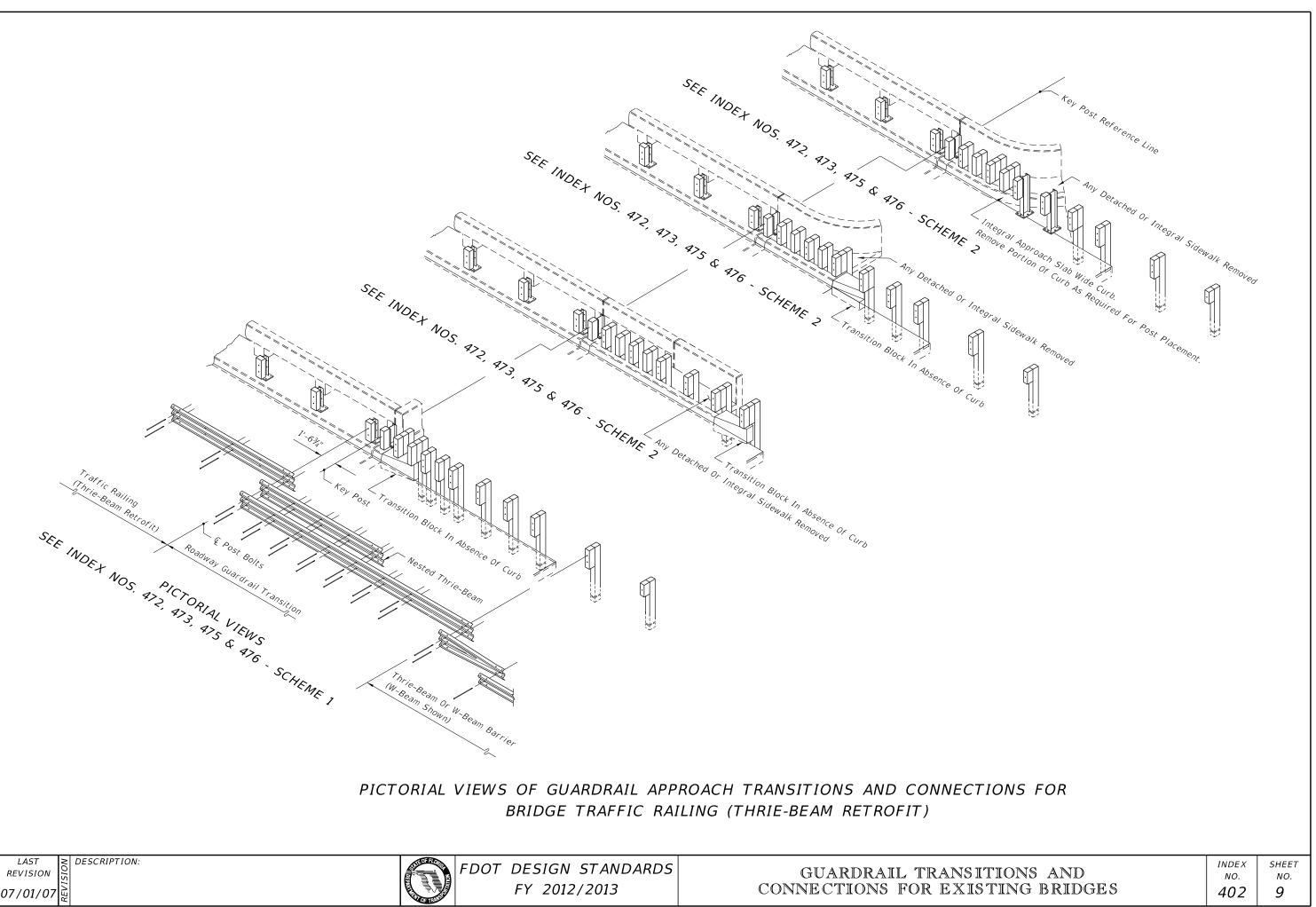


Existing Integral Approach Slab Wide Curb. Remove Portion Of Curb As Required For Post Placement. Area Of Curb Removal To Be Finished Smooth And Even With Adjoining Area.
— Special Steel Post For Roadway Thrie-Beam Transitions
Edge of Existing Approach Slab Varies Configurations Varies
Existing Approach Slab
Key Post (& Post Bolts)
Roadway Guardrail Transition
174 - SCHEME 3
Existing Integral Approach Slab Wide Curb. Remove Portion Of Curb As Required For Post Placement. Area Of Curb Removal To Be Finished Smooth And Even With Adjoining Area.
Special Steel Post For Roadway Thrie-Beam Transitions
Edge of Existing Approach Slab Varies Configurations Varies
Existing Approach Slab
Key Post (© Post Bolts) Roadway Guardrail Transition
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174 - SCHEME 3
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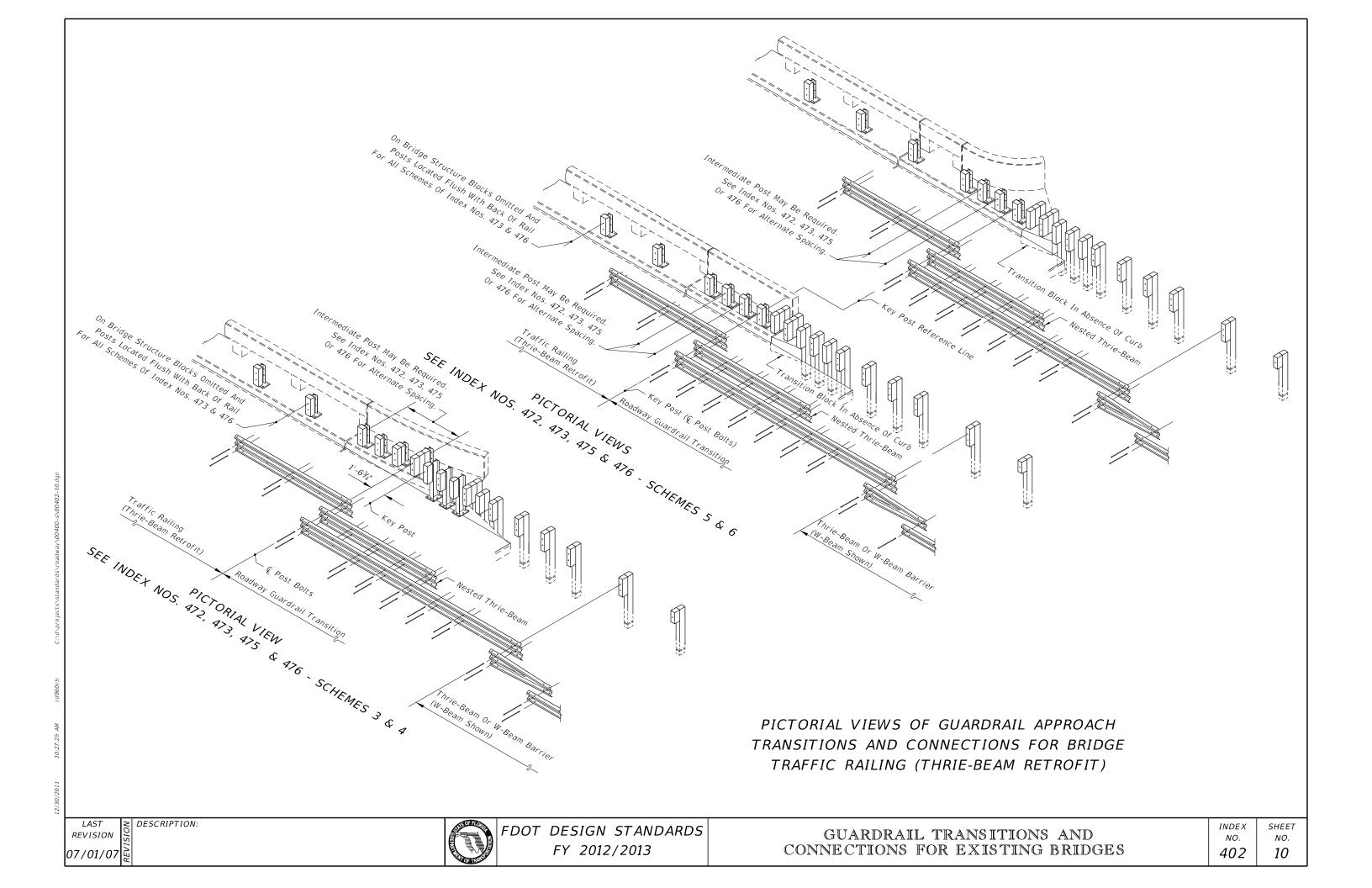


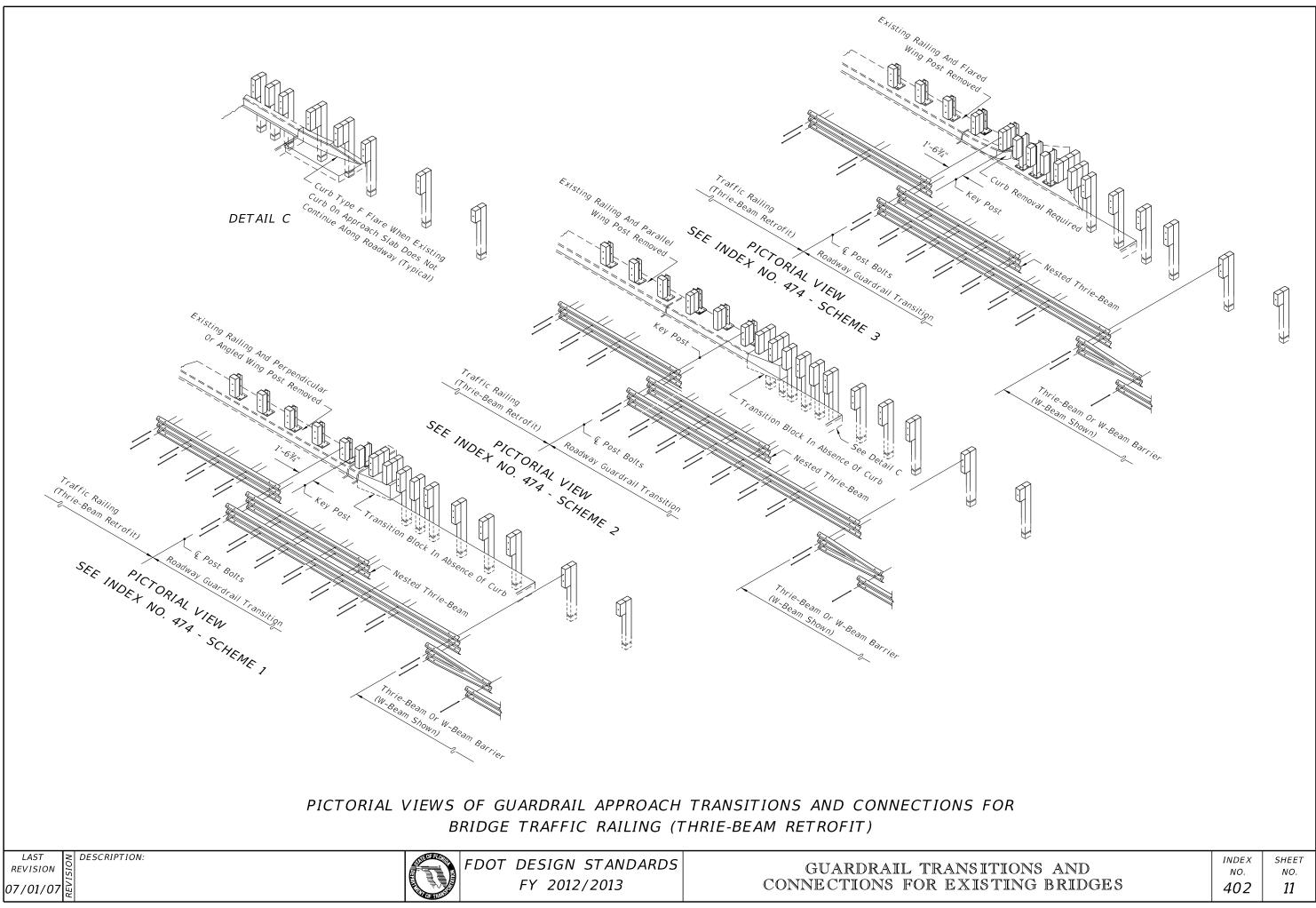
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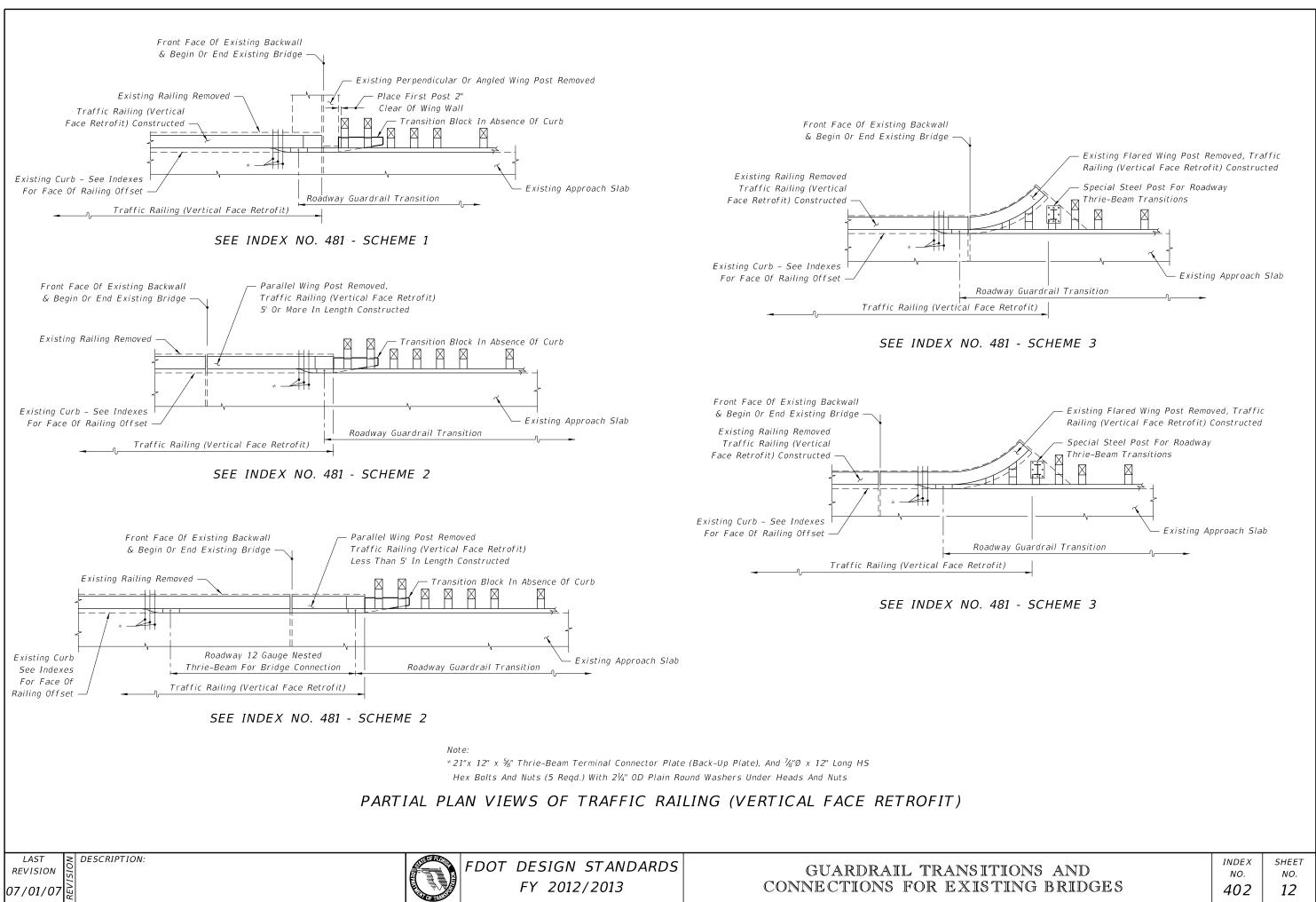
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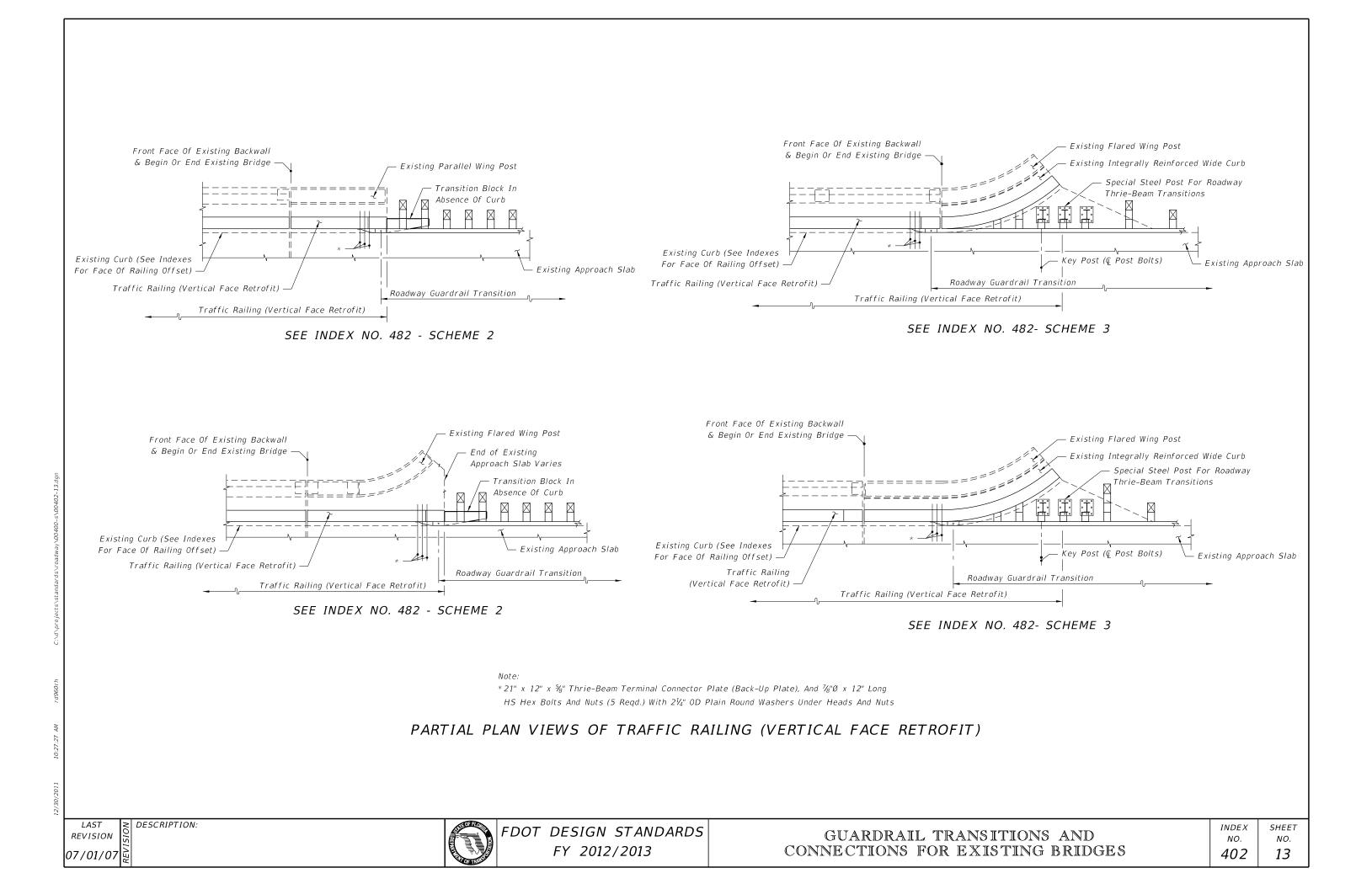


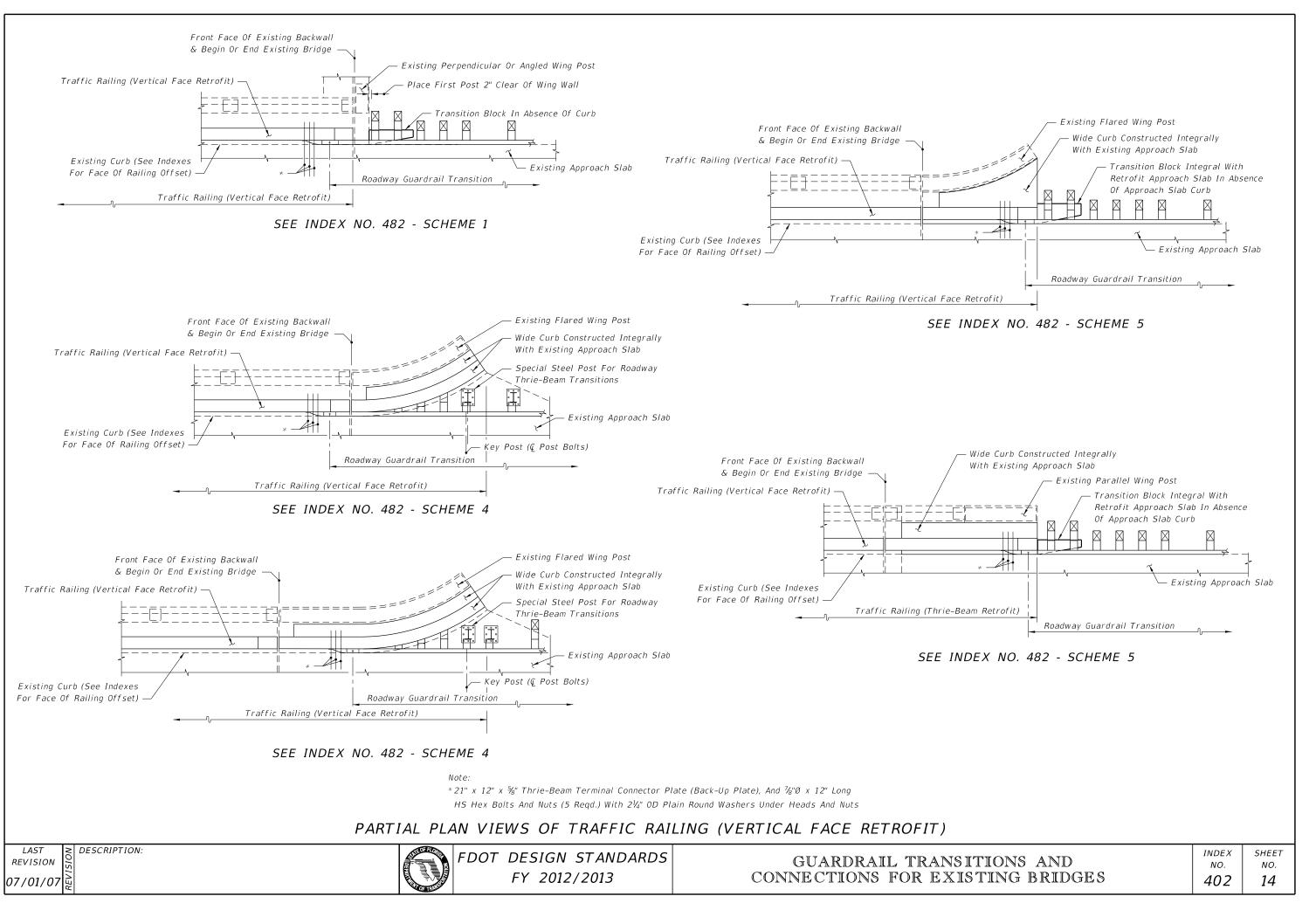


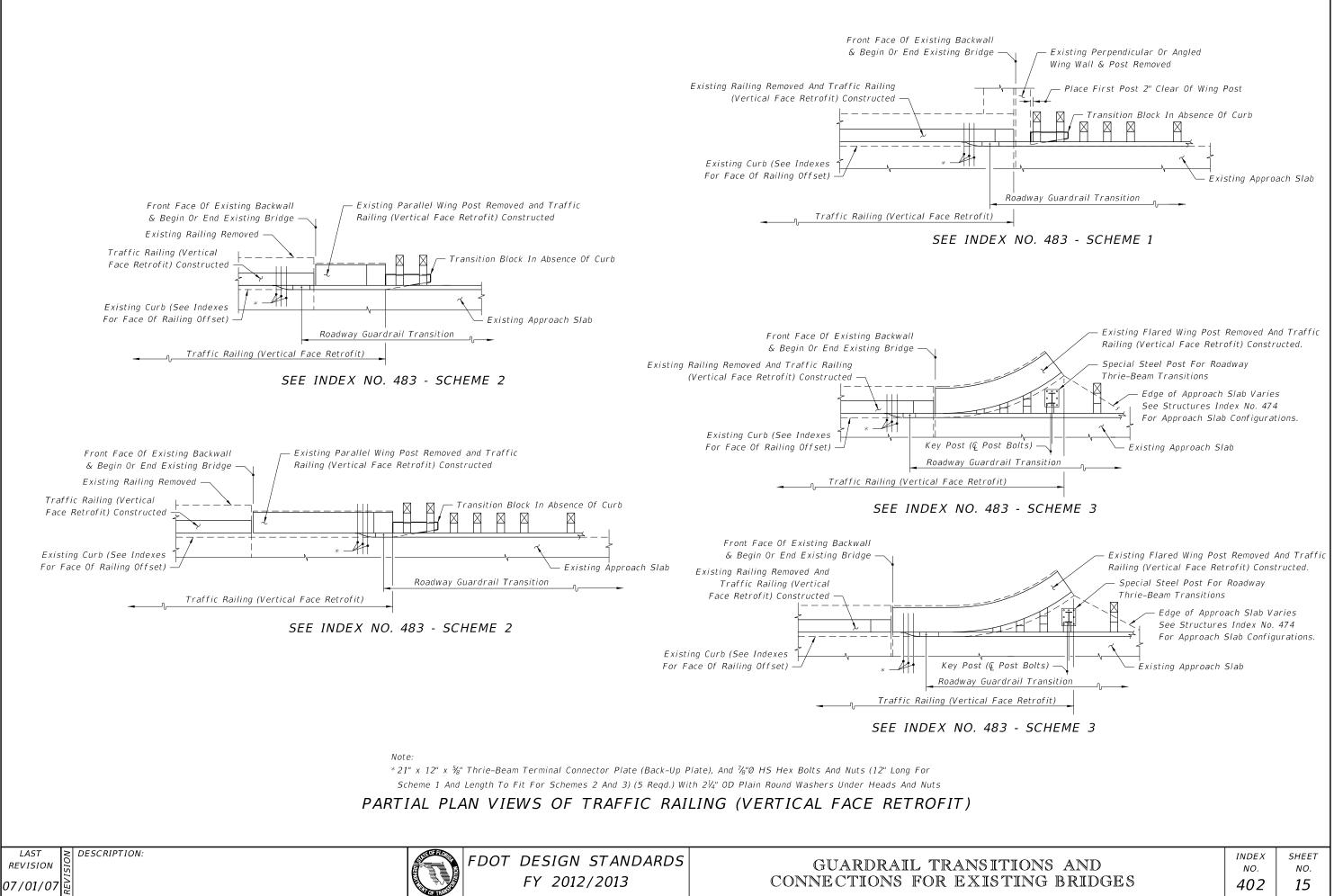
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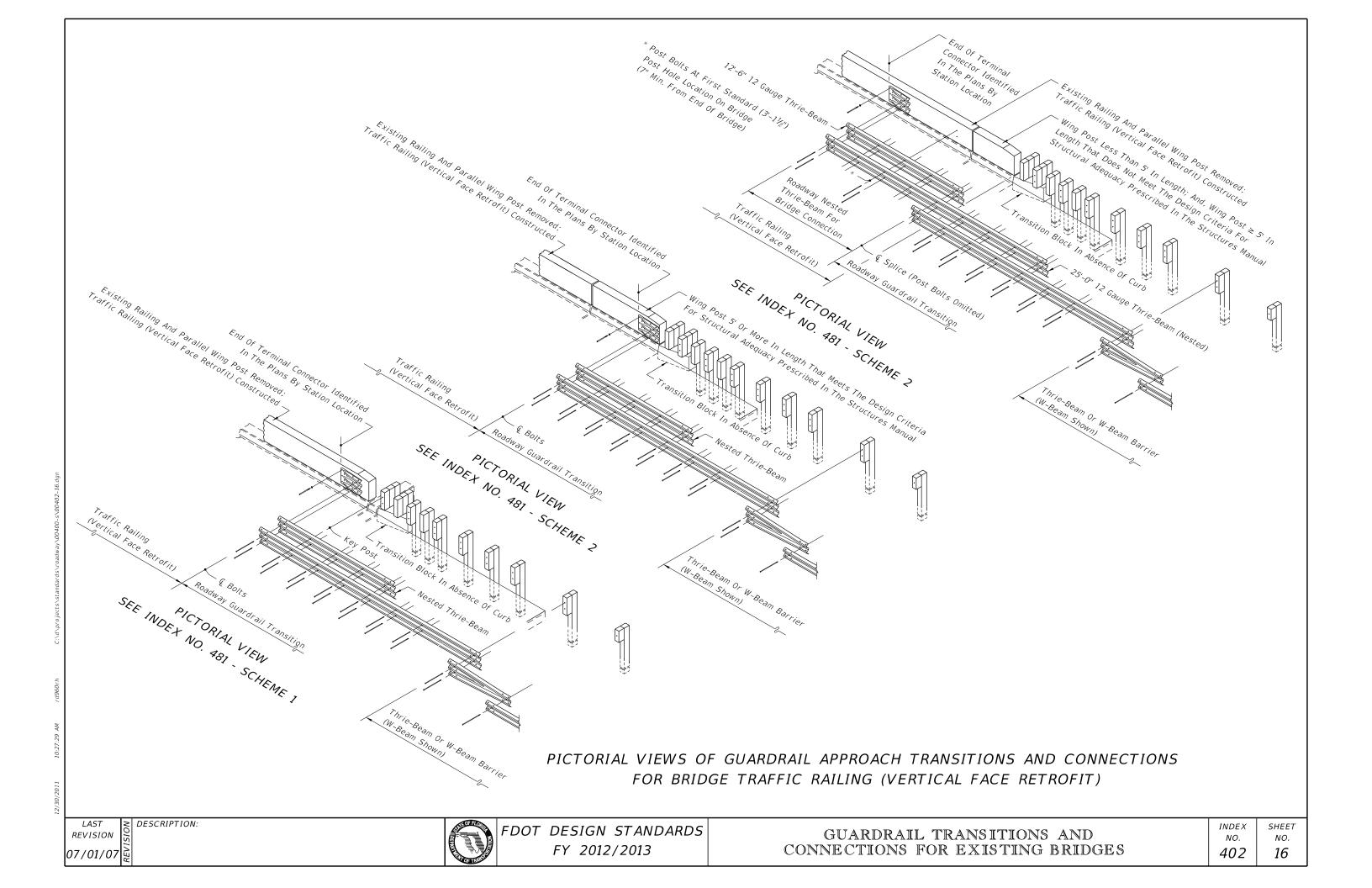


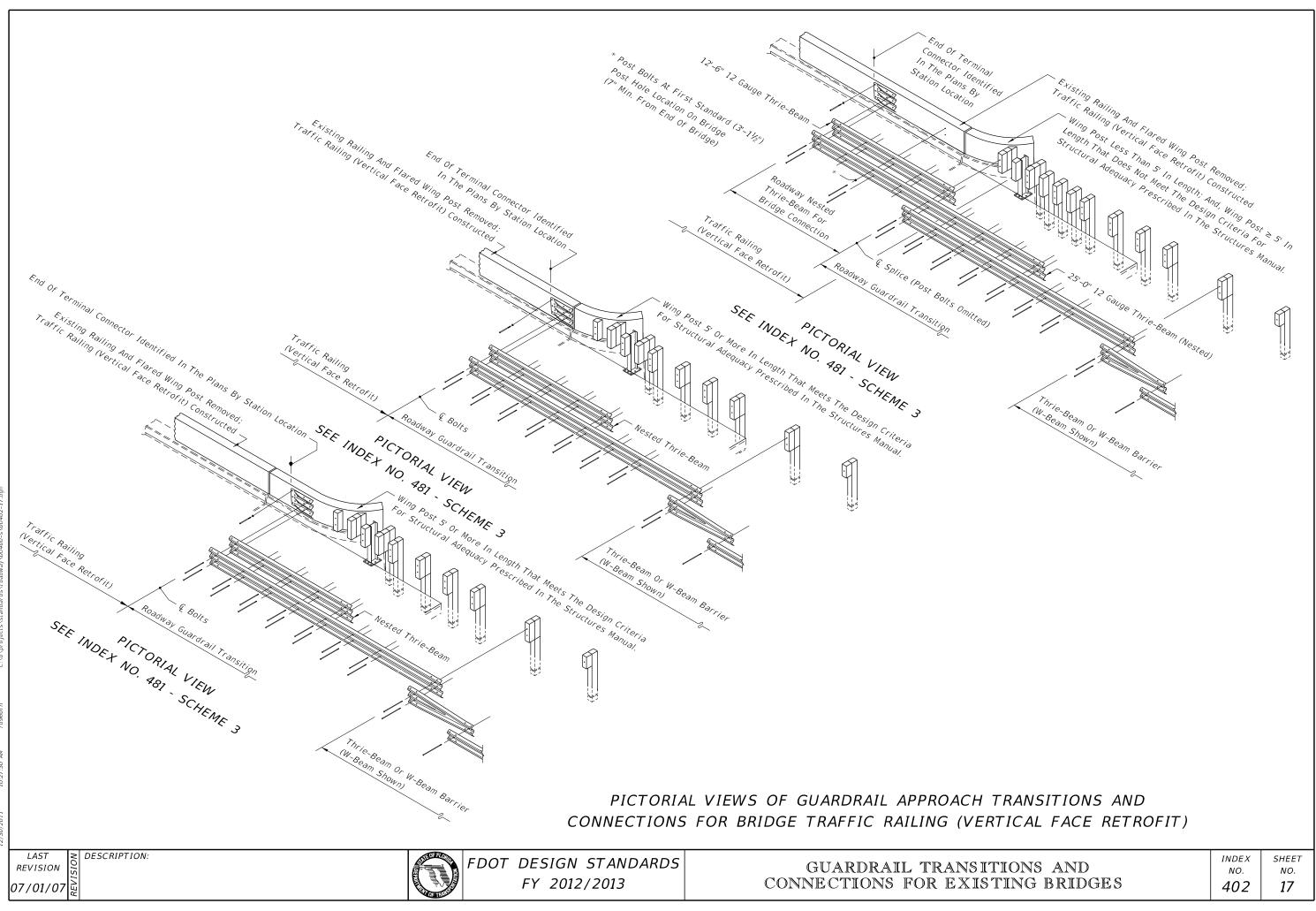


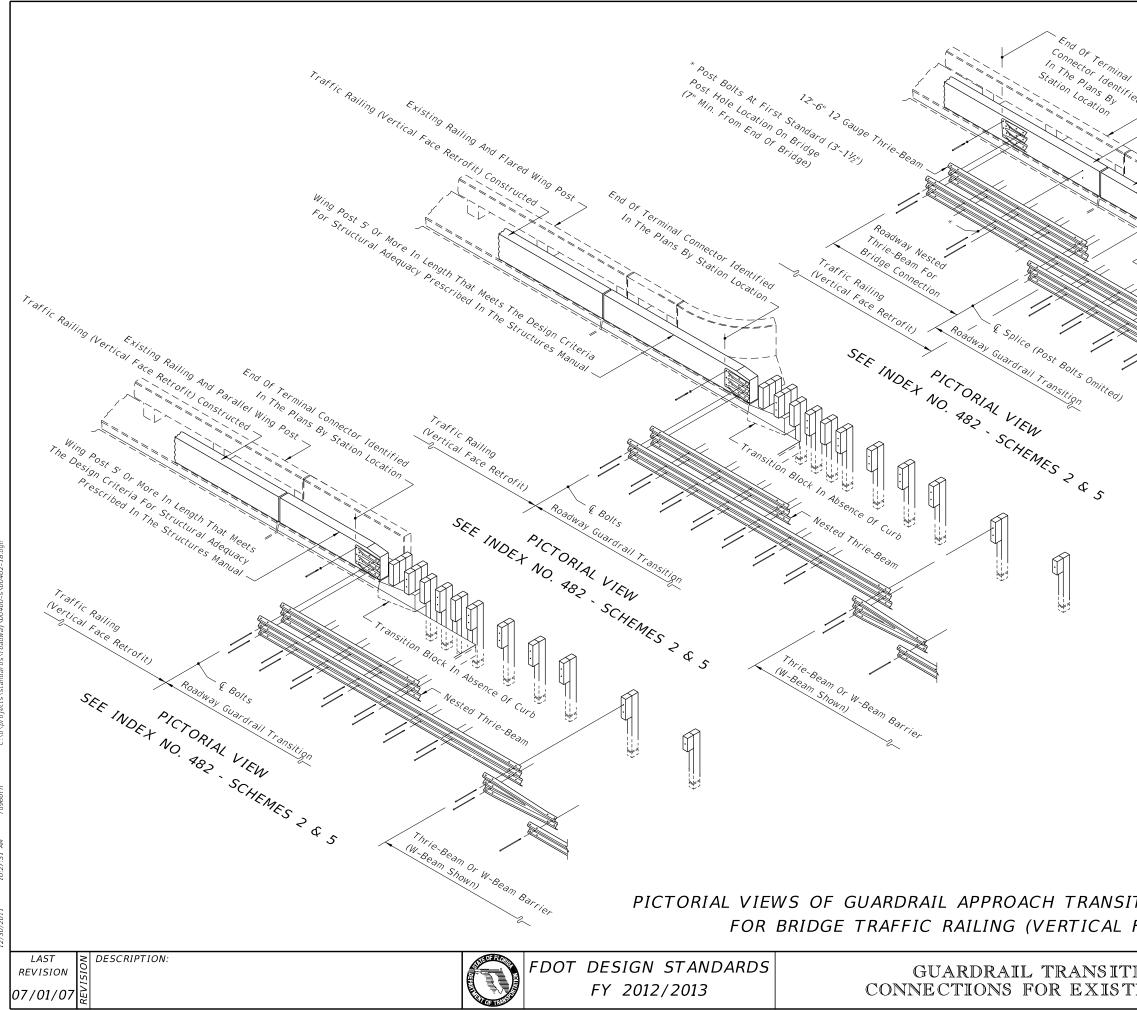




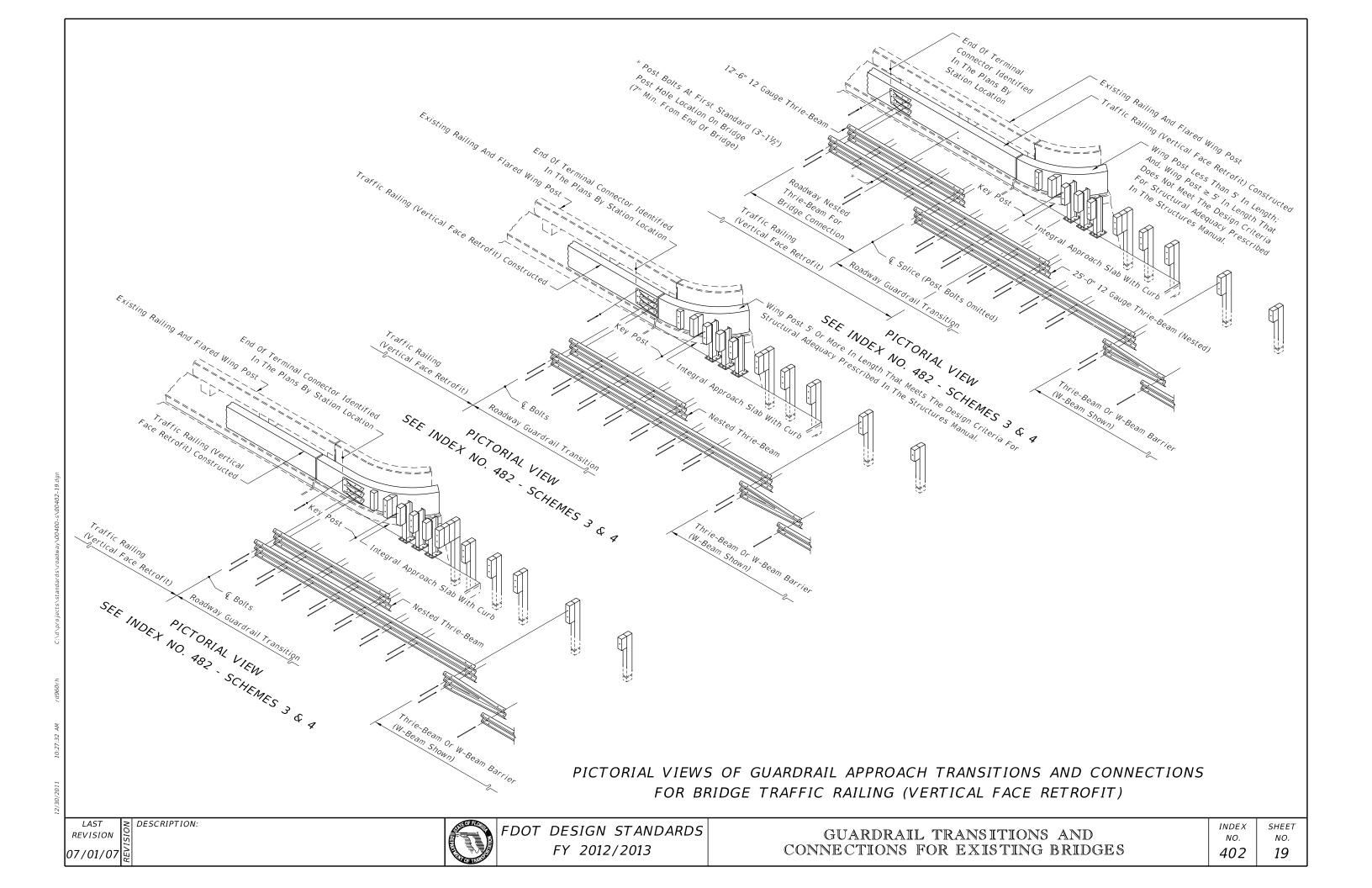


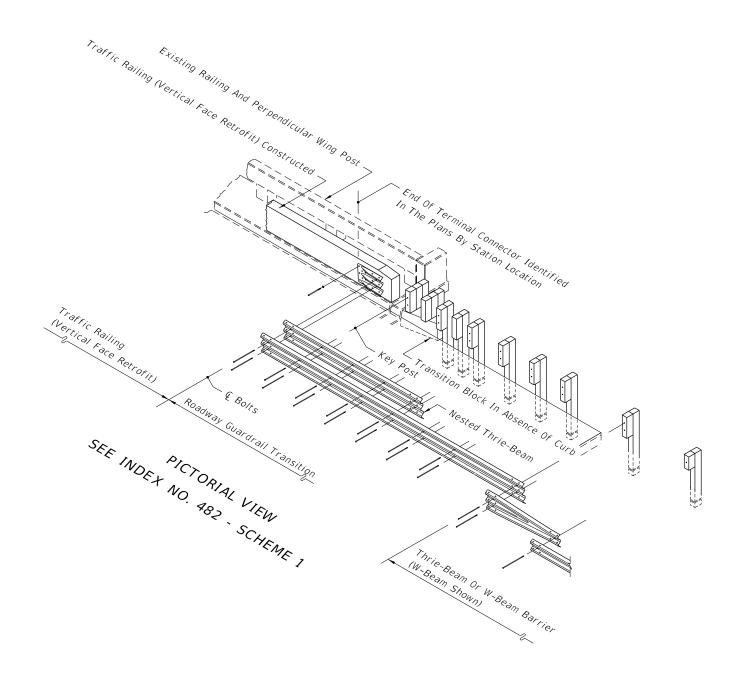






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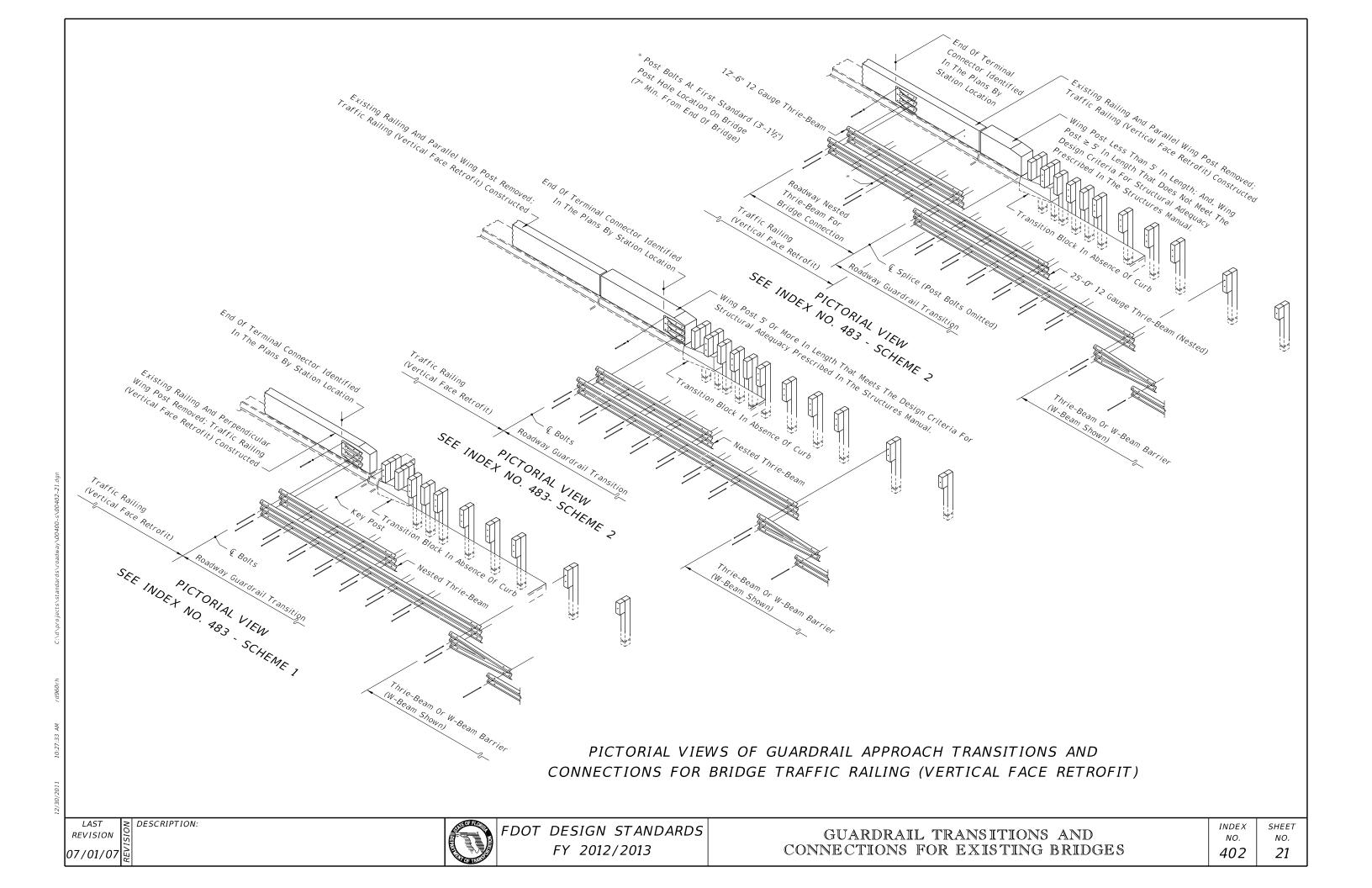
PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT)

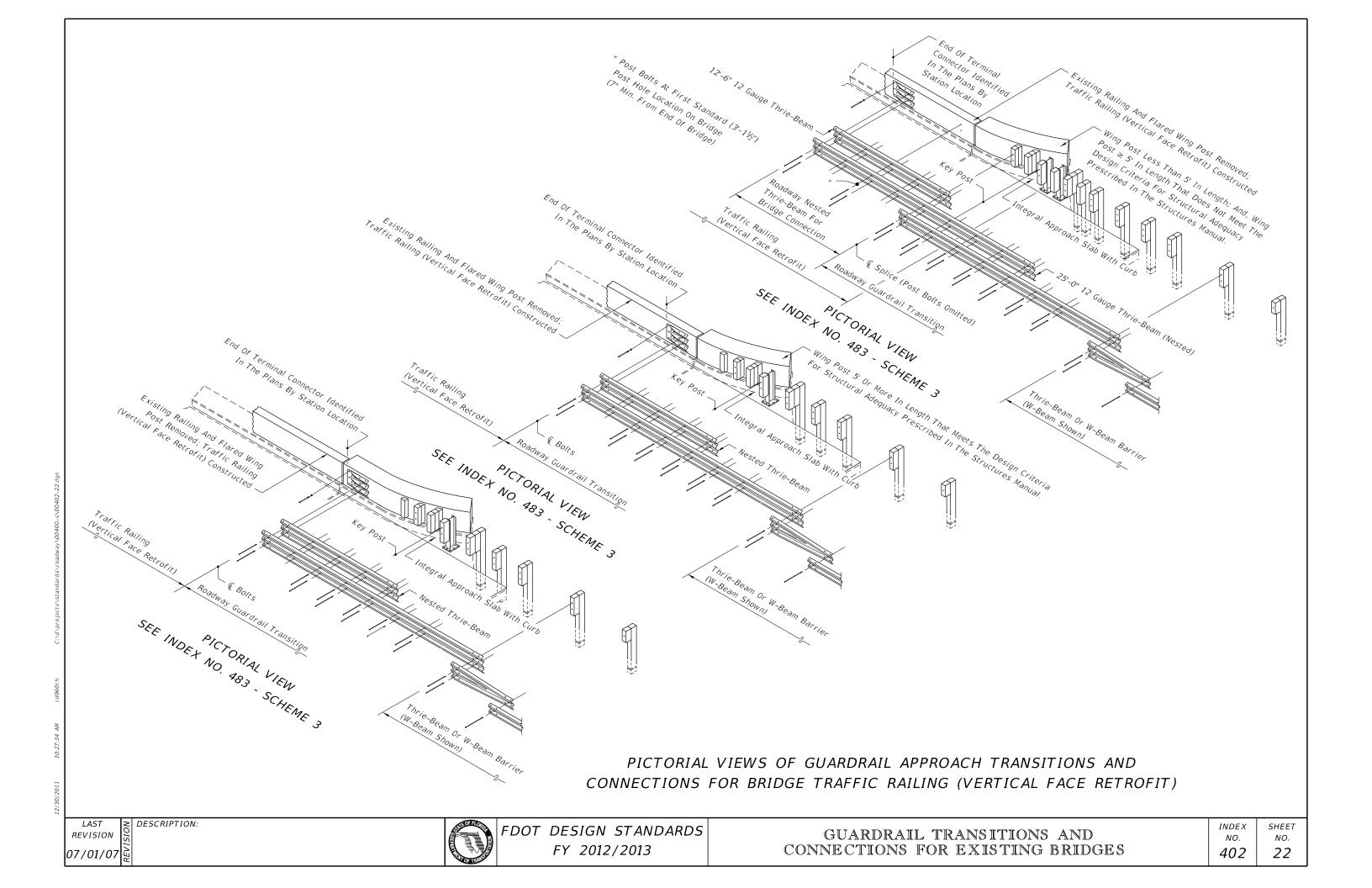


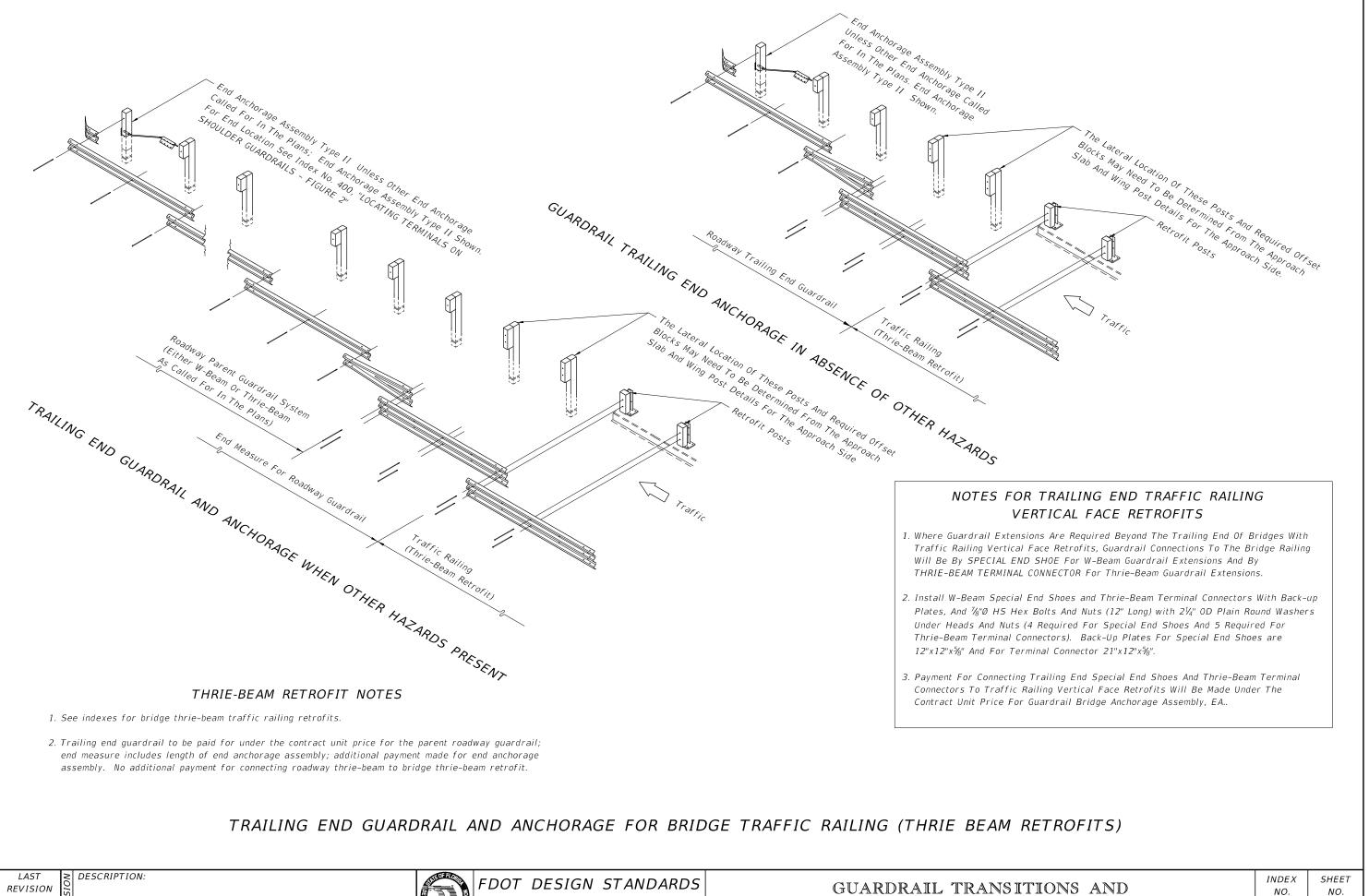


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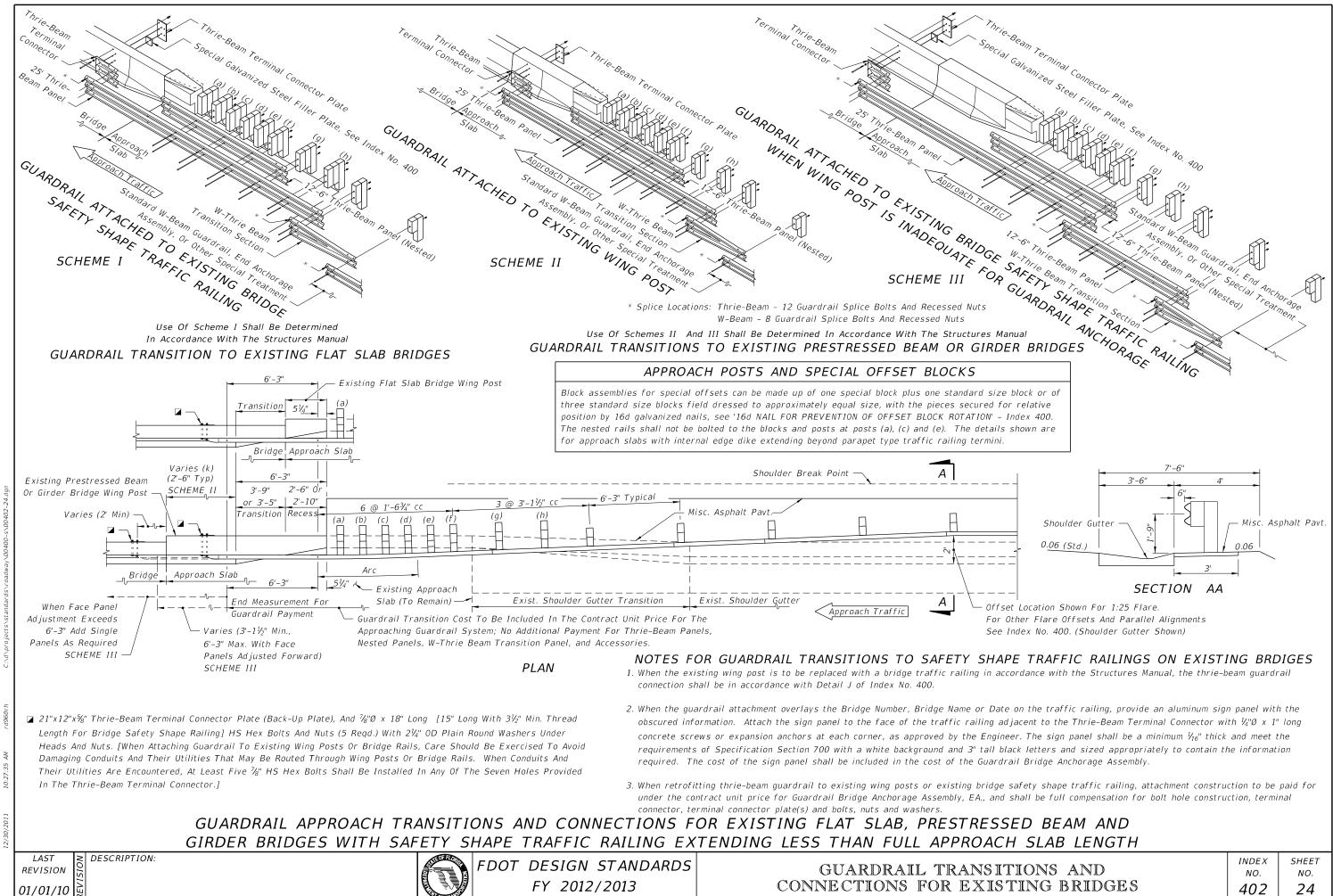


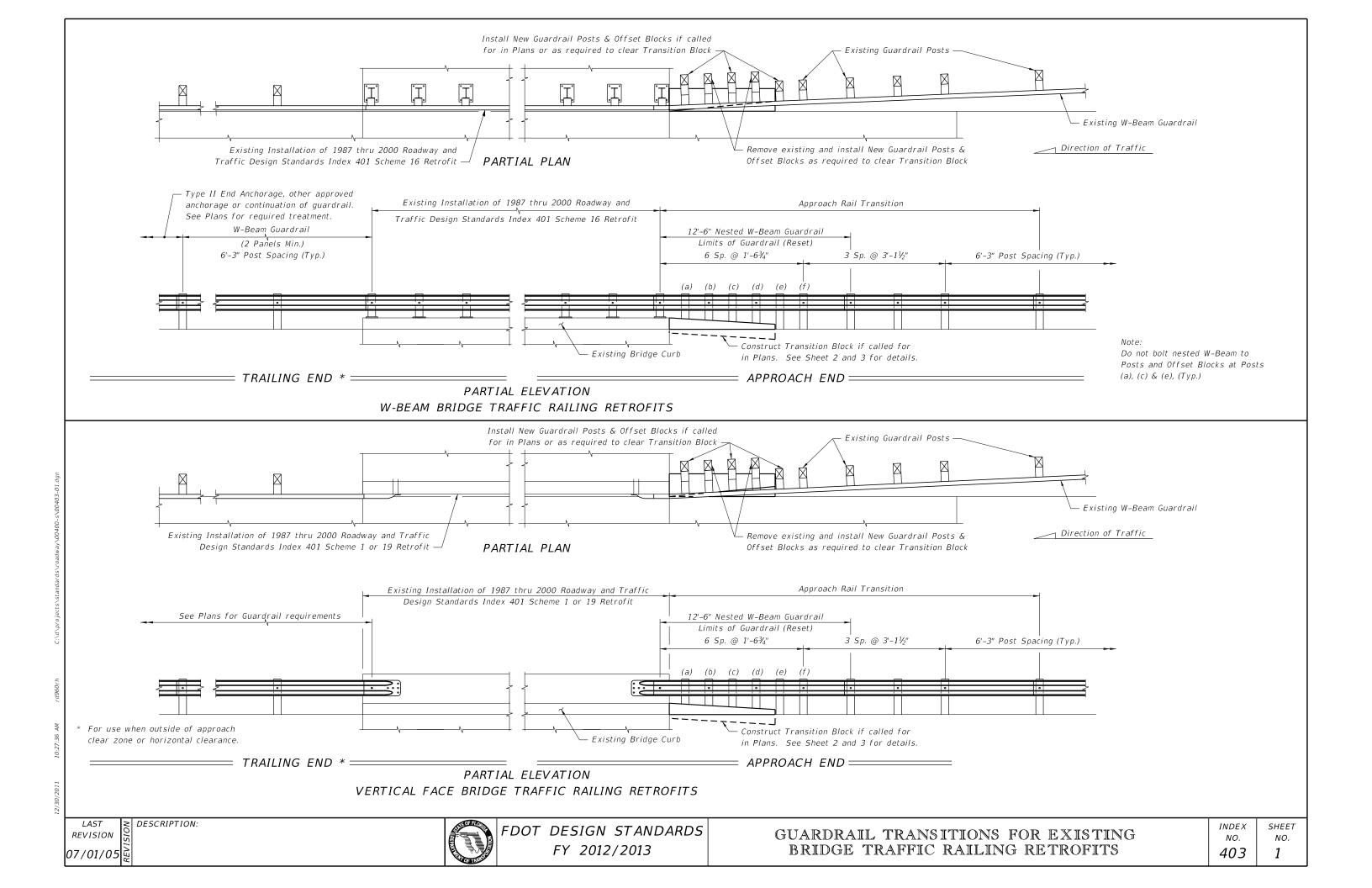


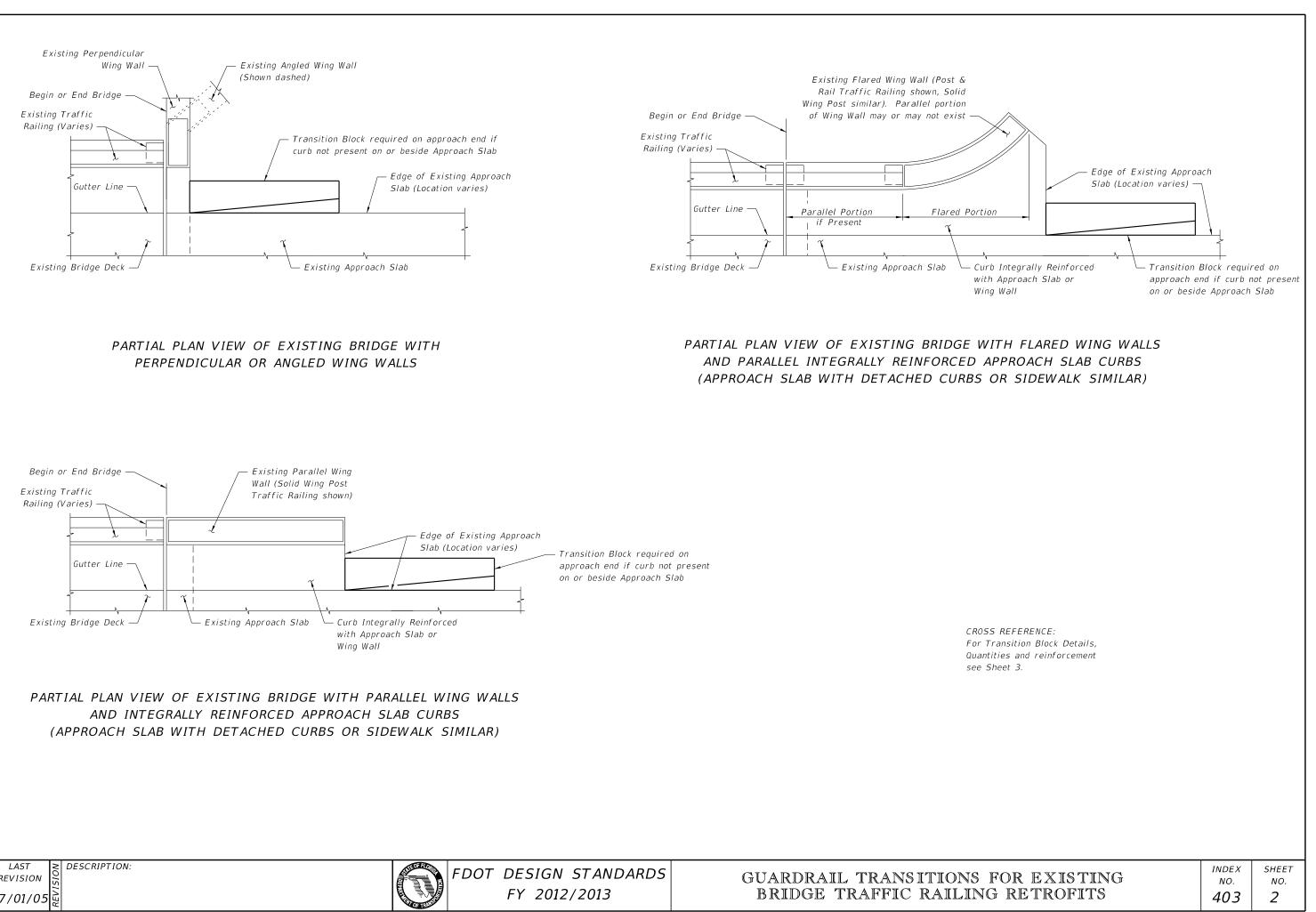


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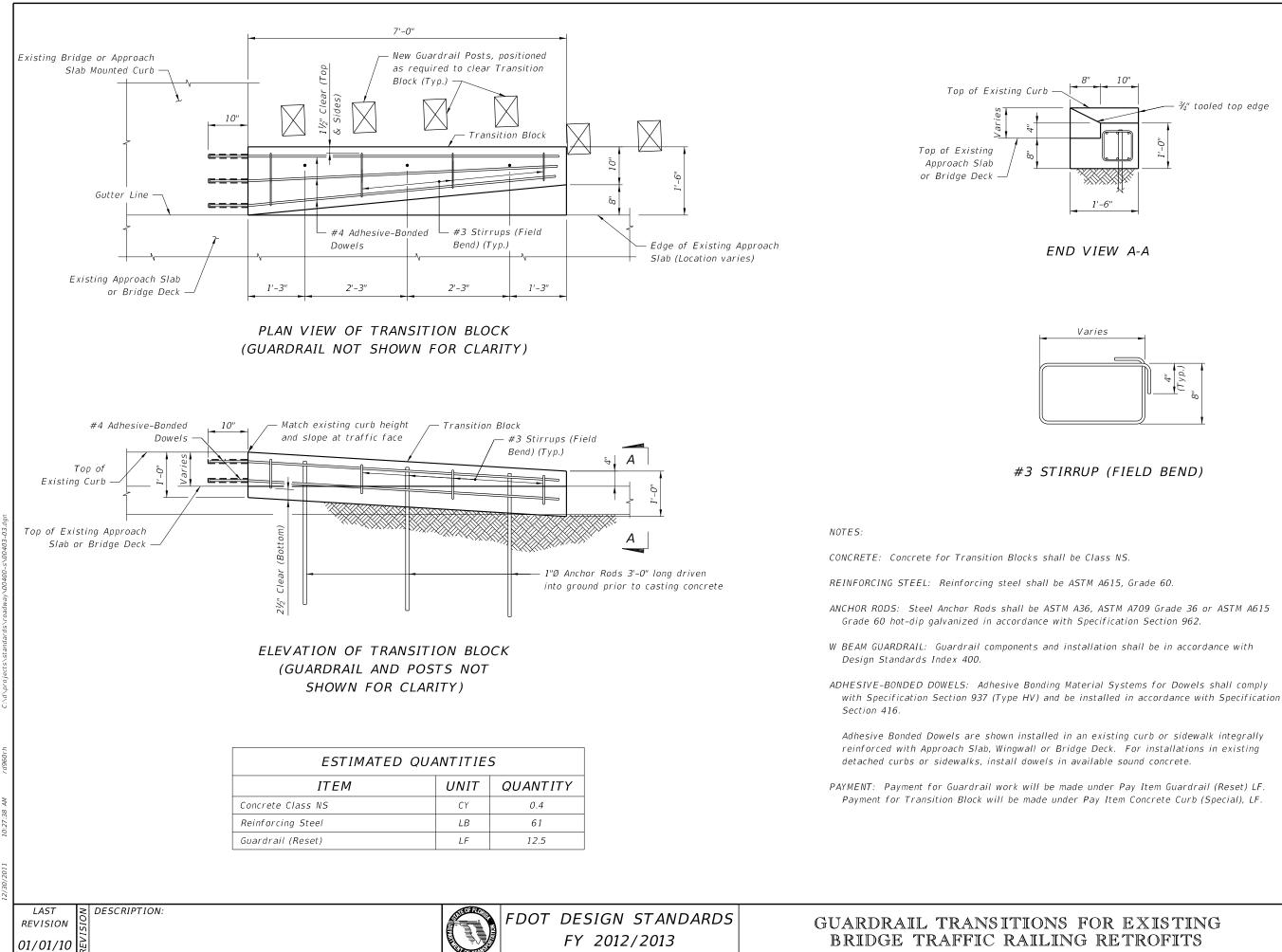




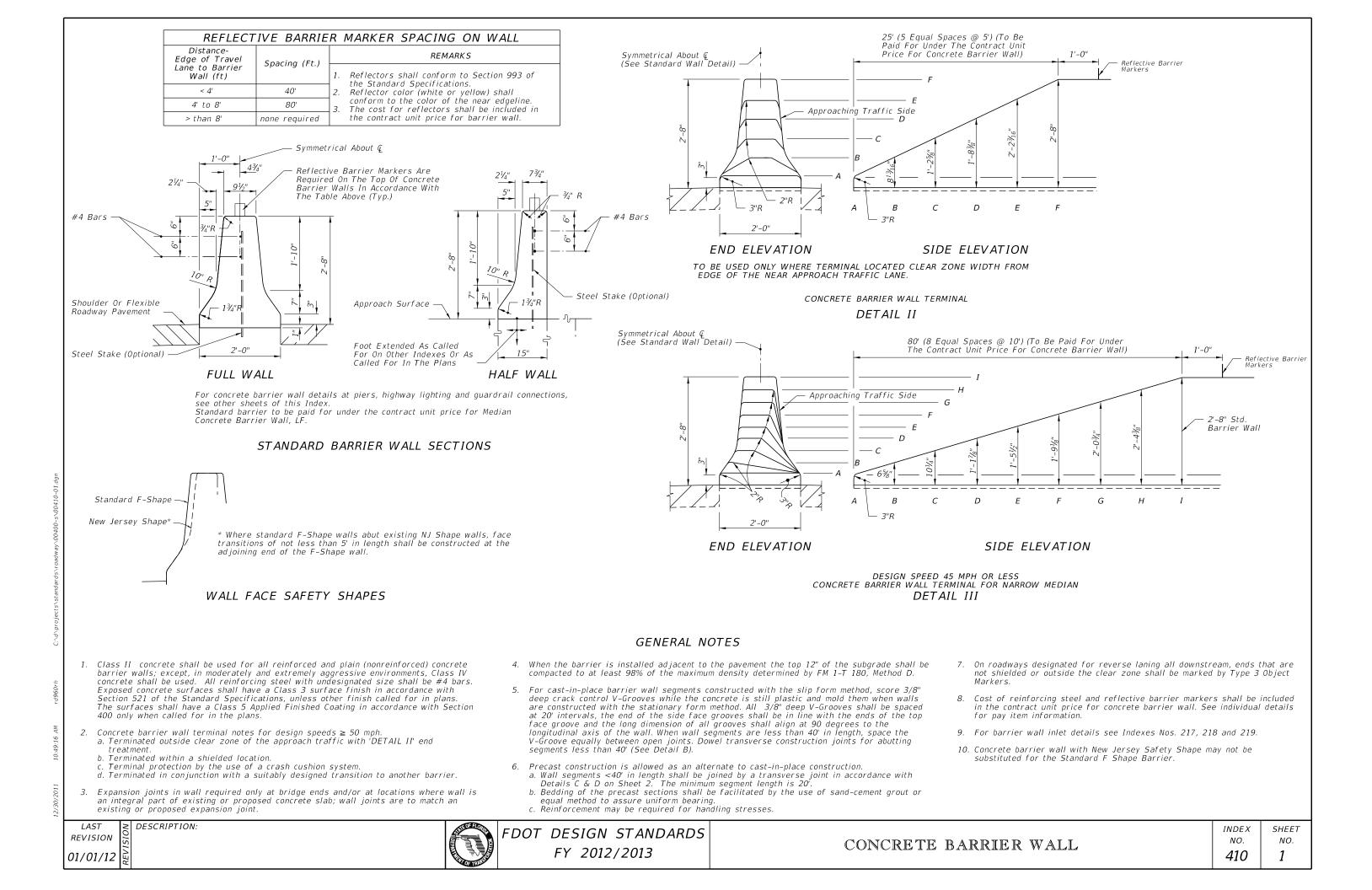


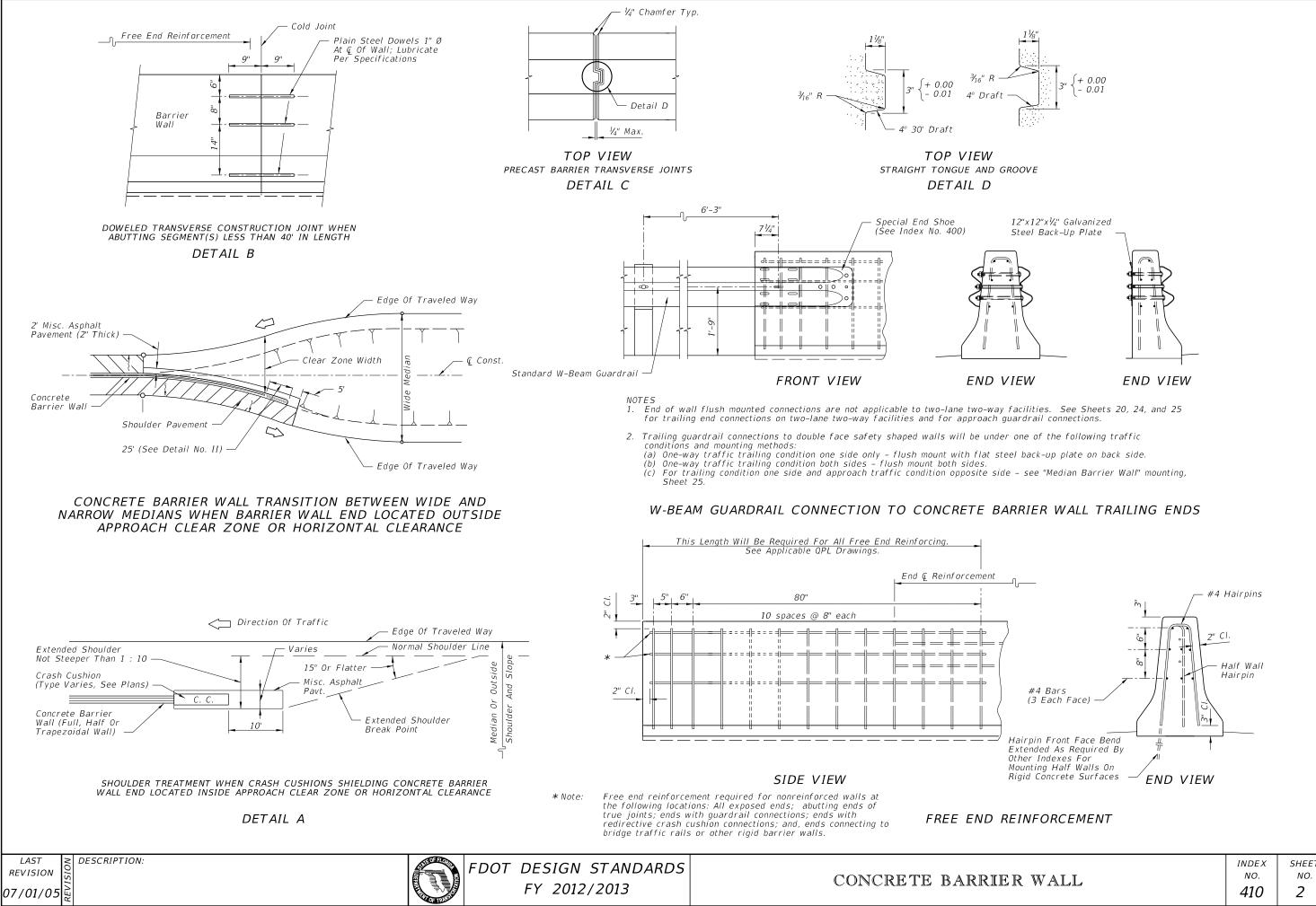


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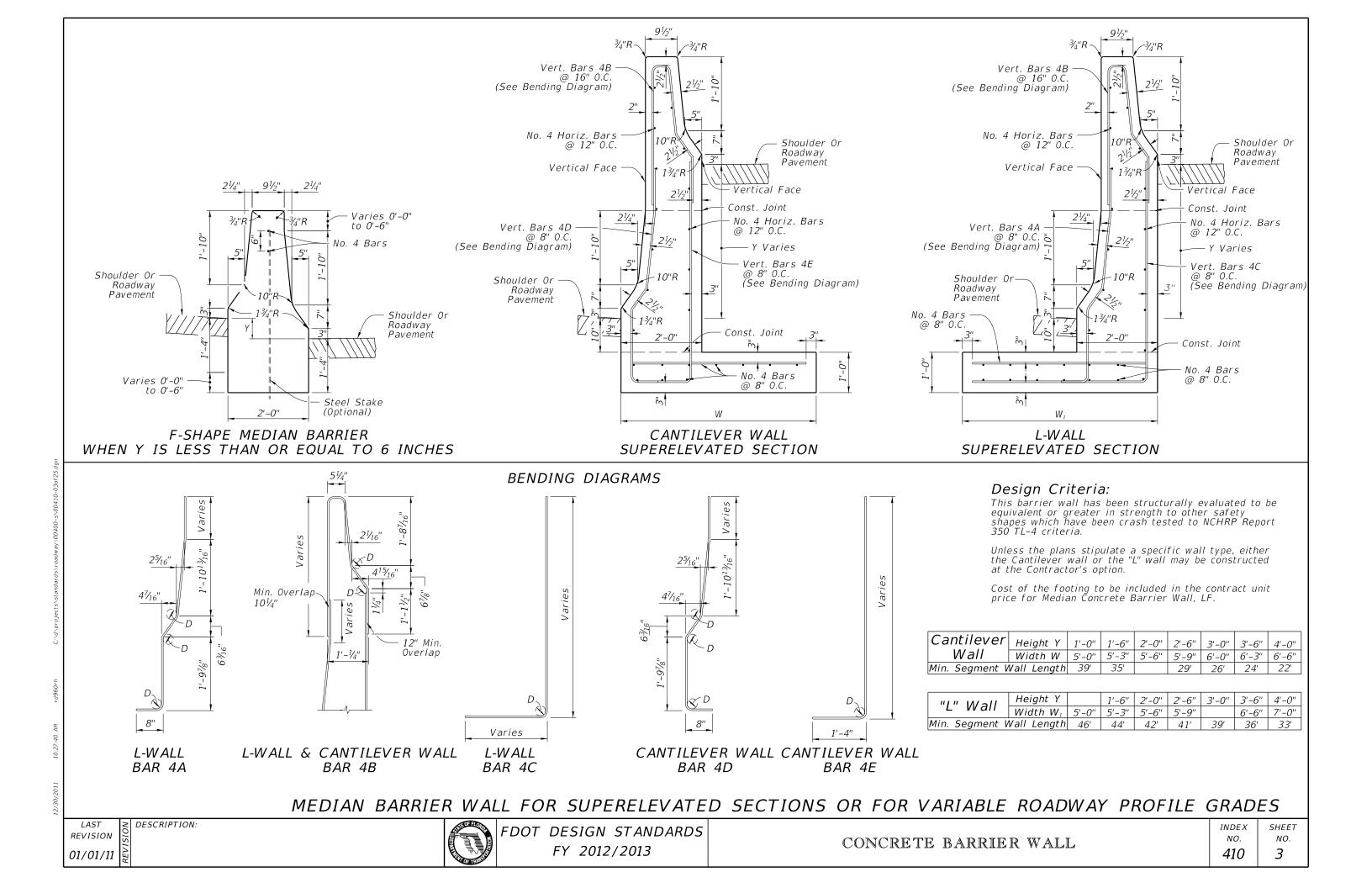


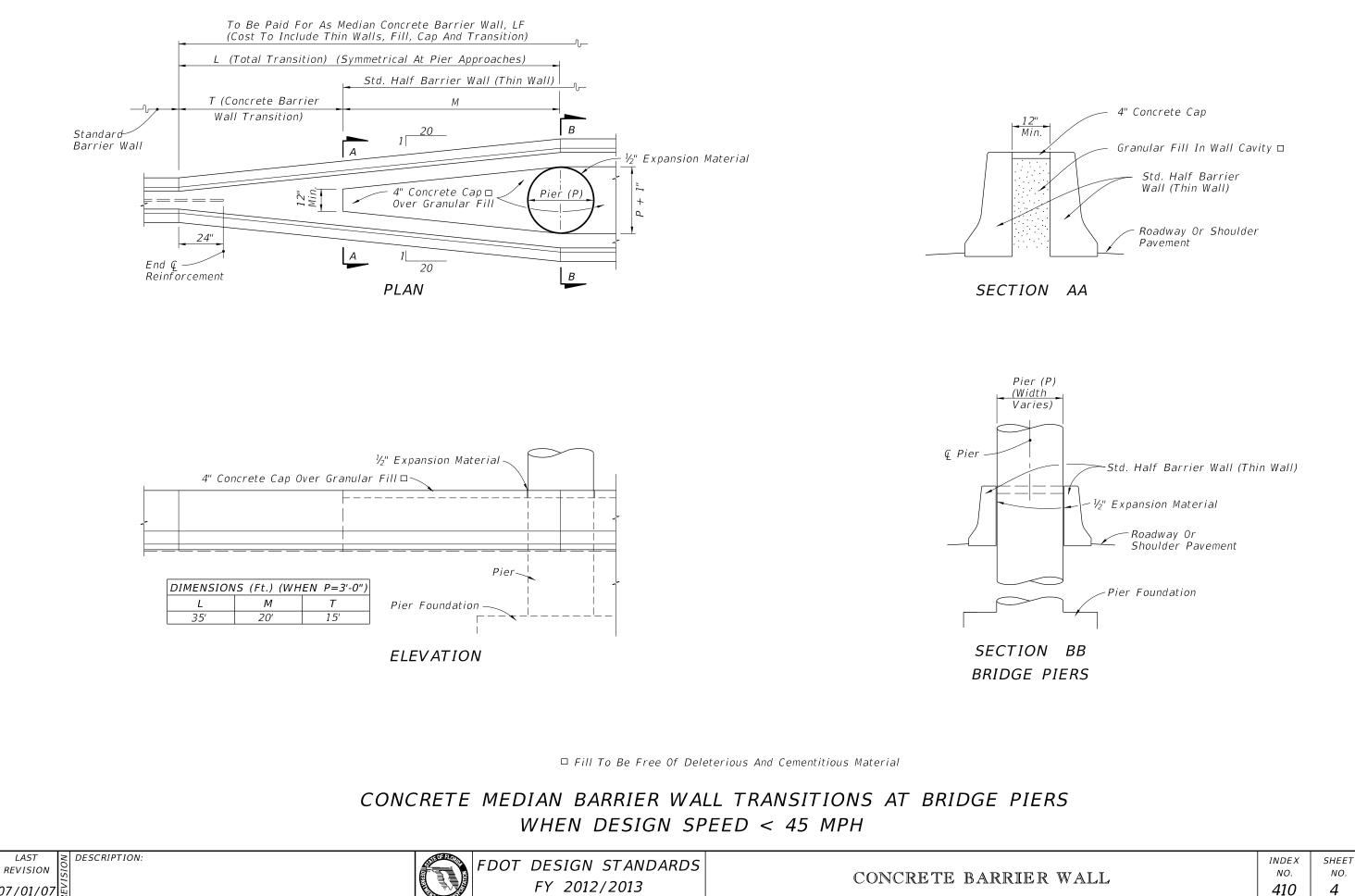
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G RETROFITS	403	3





Hairpin Front Face Bend Extended As Required By Other Indexes For Mounting Half Walls On Rigid Concrete Surfaces END VIEW		
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ER WALL	index no. <b>410</b>	sнеет NO. <b>2</b>

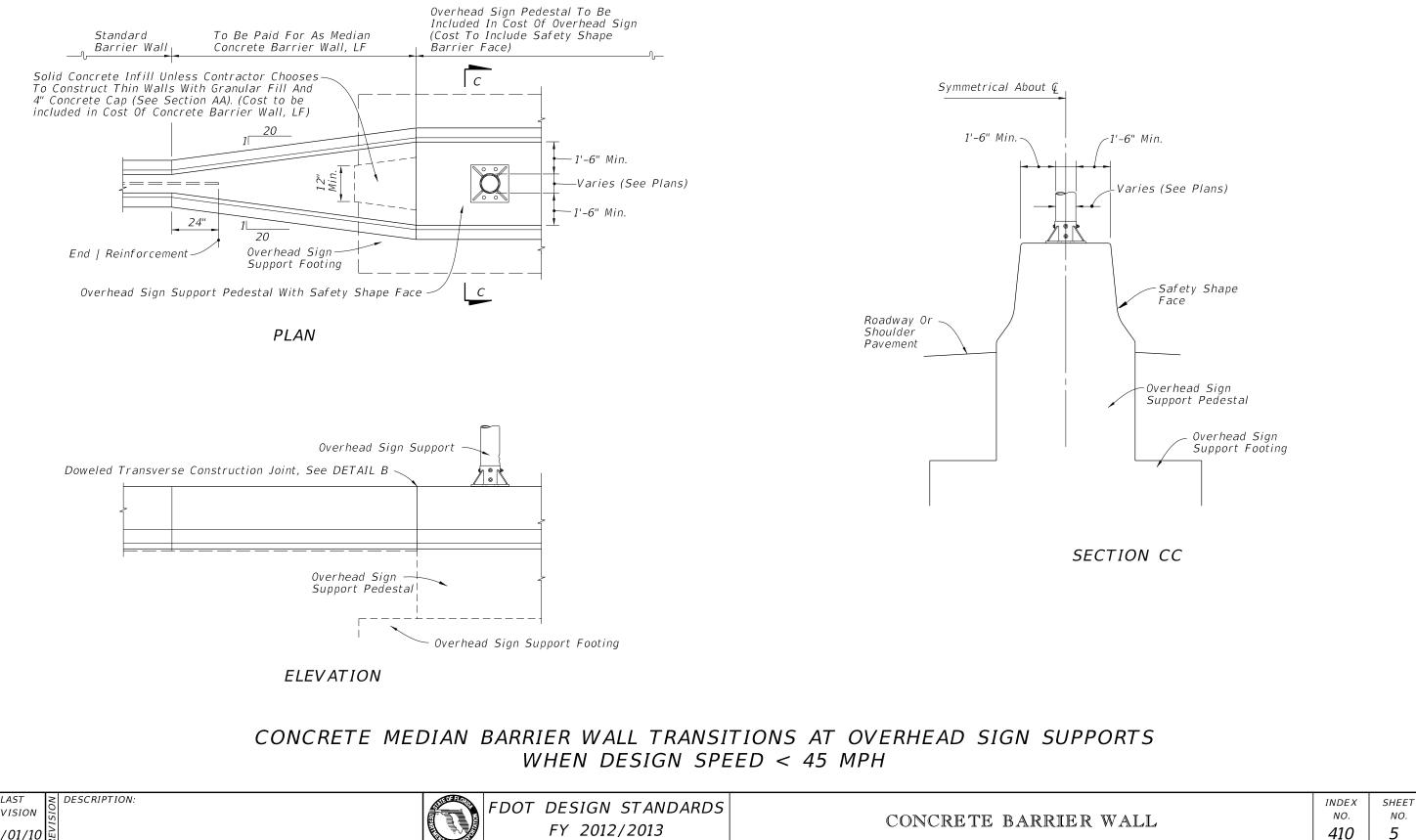




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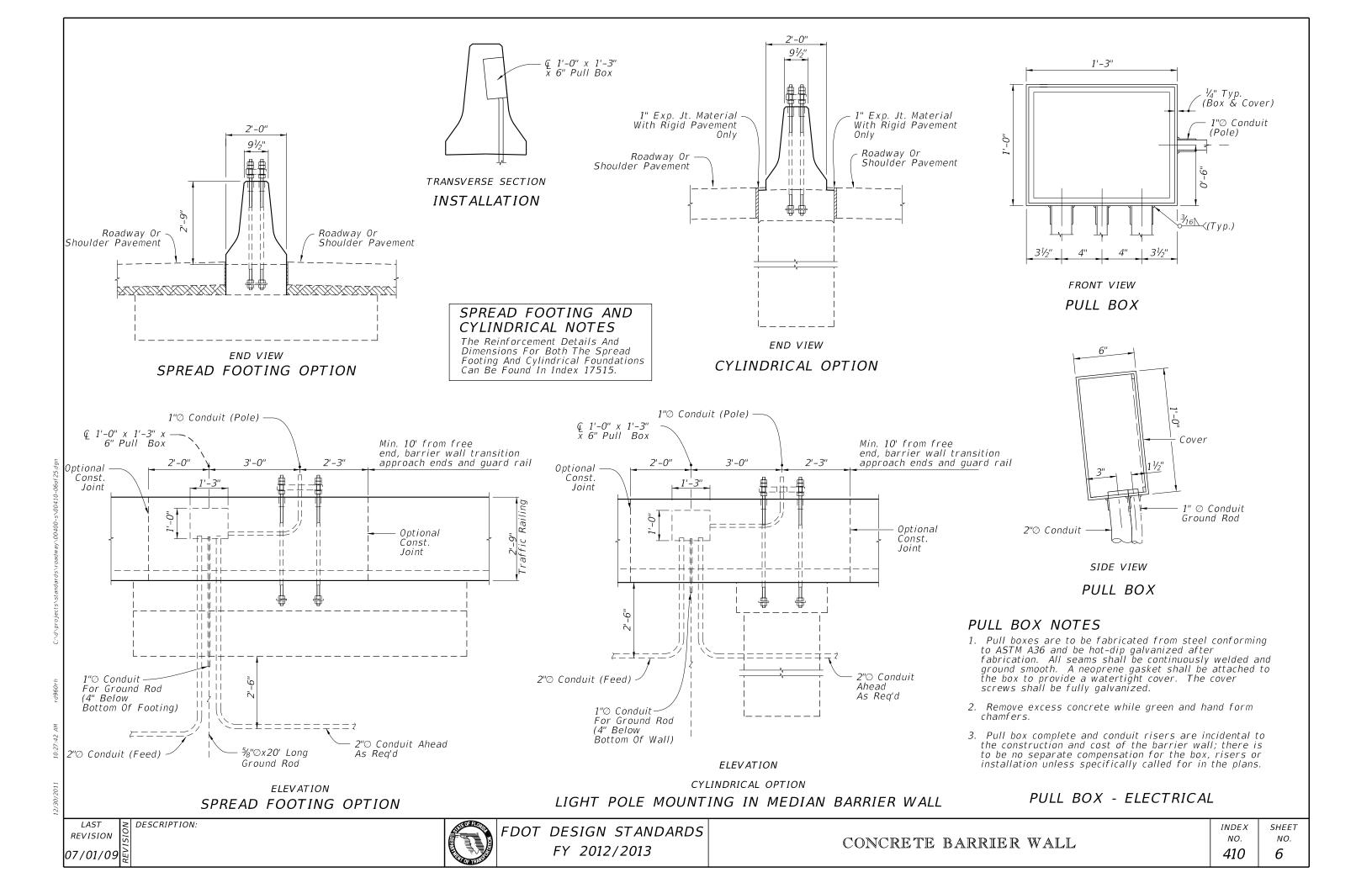
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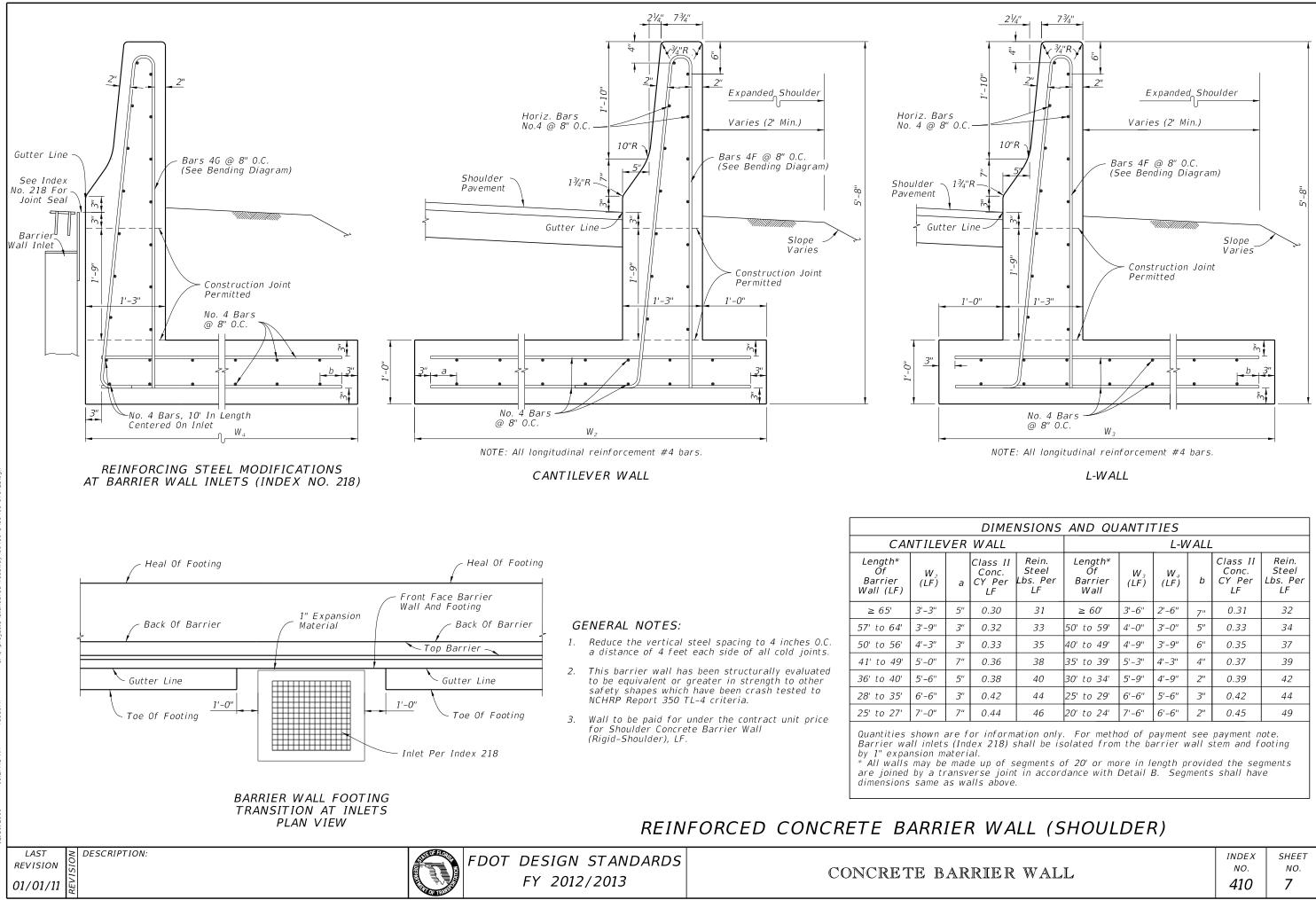
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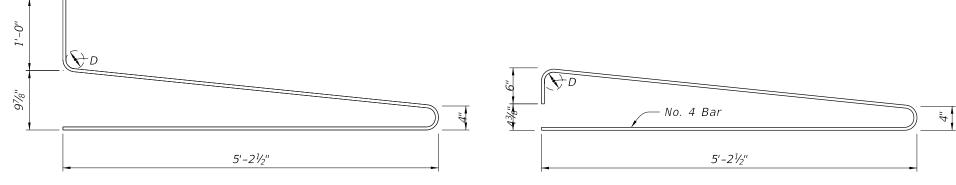






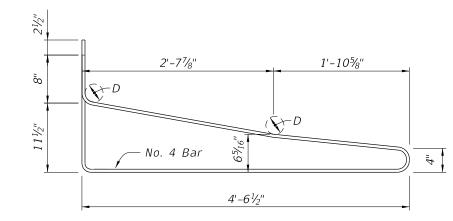
IENSIONS AND QUANTITIES							
L L-WALL							
II :. er	Rein. Steel Lbs. Per LF	Length* Of Barrier Wall	₩₃ (LF)	W4 (LF)	b	Class II Conc. CY Per LF	Rein. Steel Lbs. Per LF
)	31	≥ 60′	3'-6"	2'-6"	7"	0.31	32
2	33	50' to 59'	4'-0''	3'-0"	5"	0.33	34
}	35	40' to 49'	4'-9''	3'-9"	6"	0.35	37
i	38	35' to 39'	5'-3''	4'-3"	4''	0.37	39
?	40	30' to 34'	5'-9"	4'-9''	2"	0.39	42
)	44	25' to 29'	6'-6"	5'-6"	3"	0.42	44
	46	20' to 24'	7'-6"	6'-6"	2"	0.45	49

	INDEX	SHEET
R WALL	NO.	NO.
	410	7

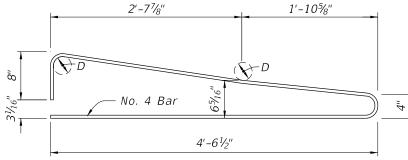


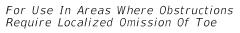




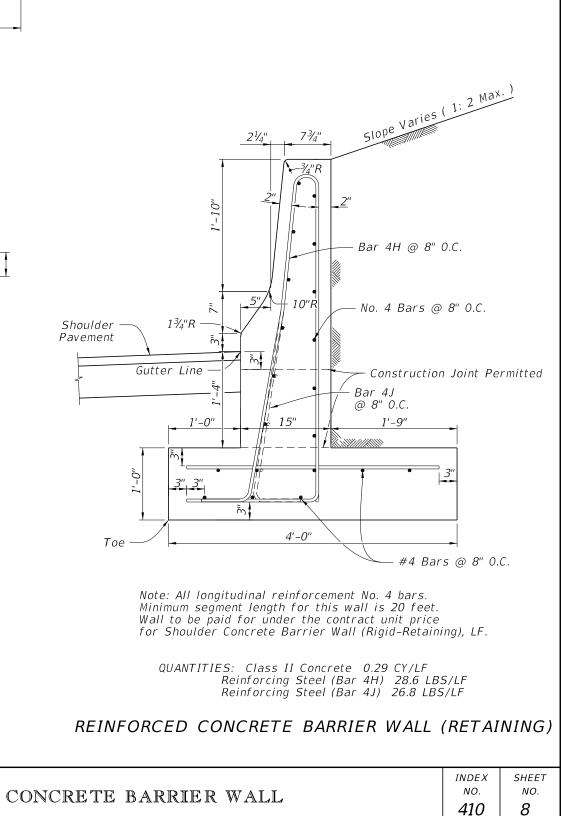


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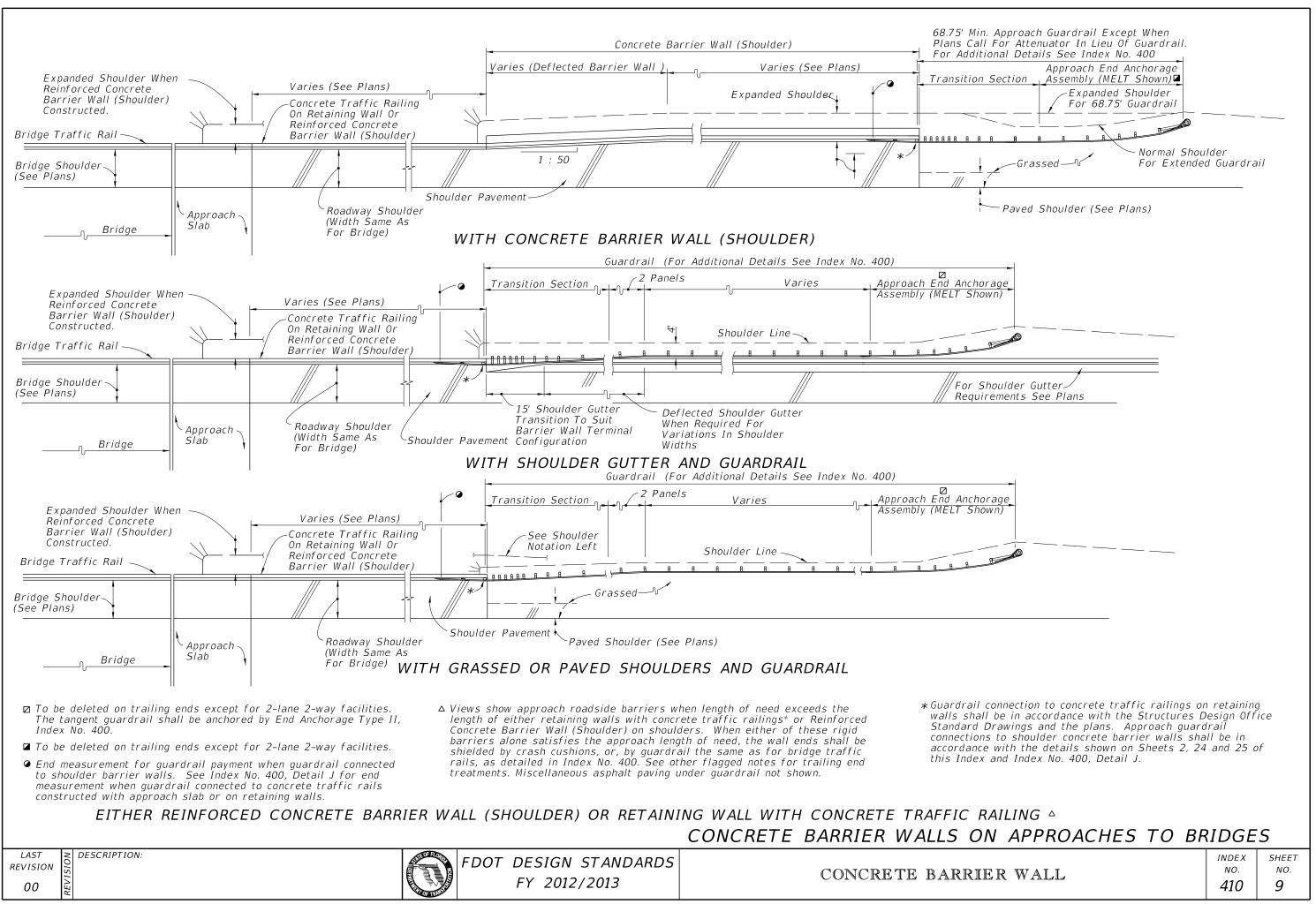


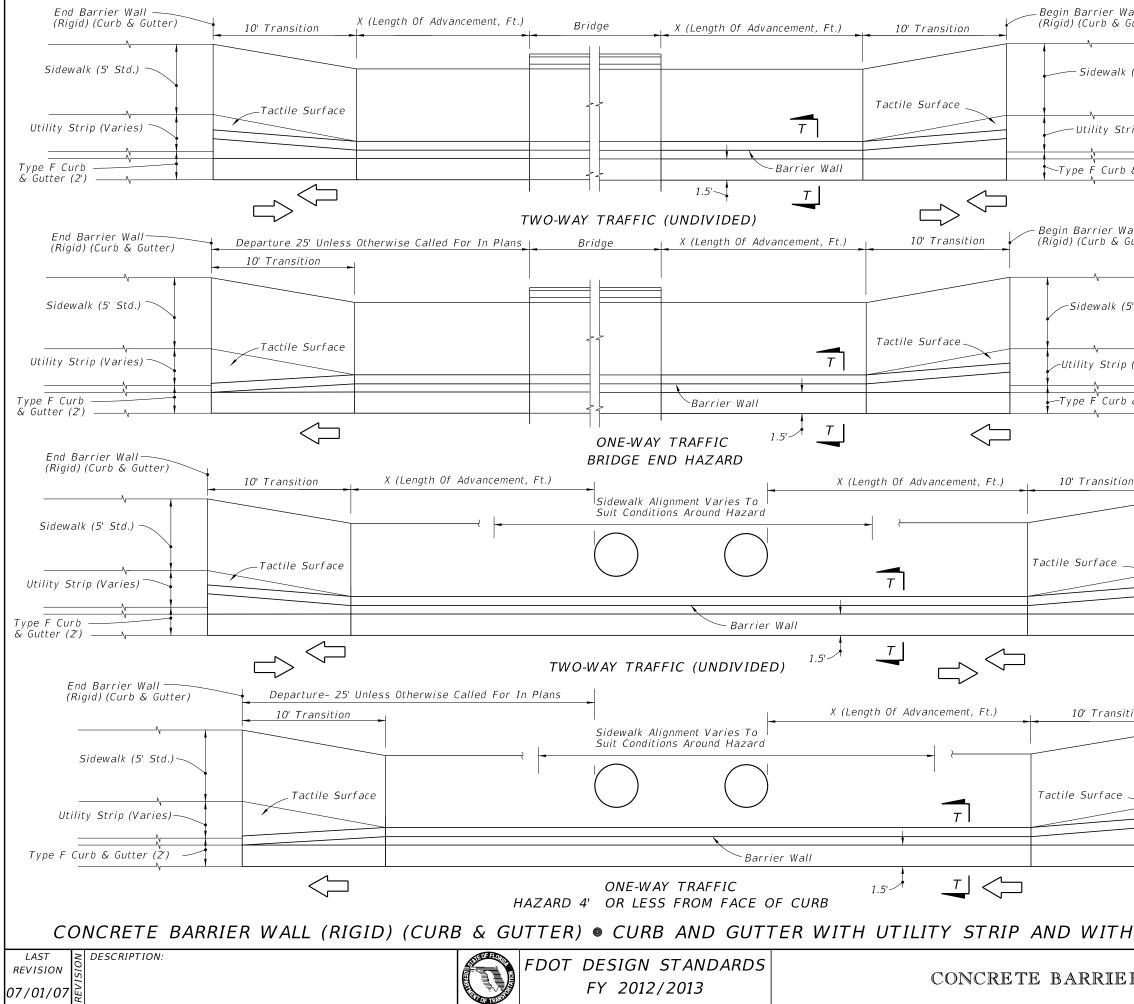


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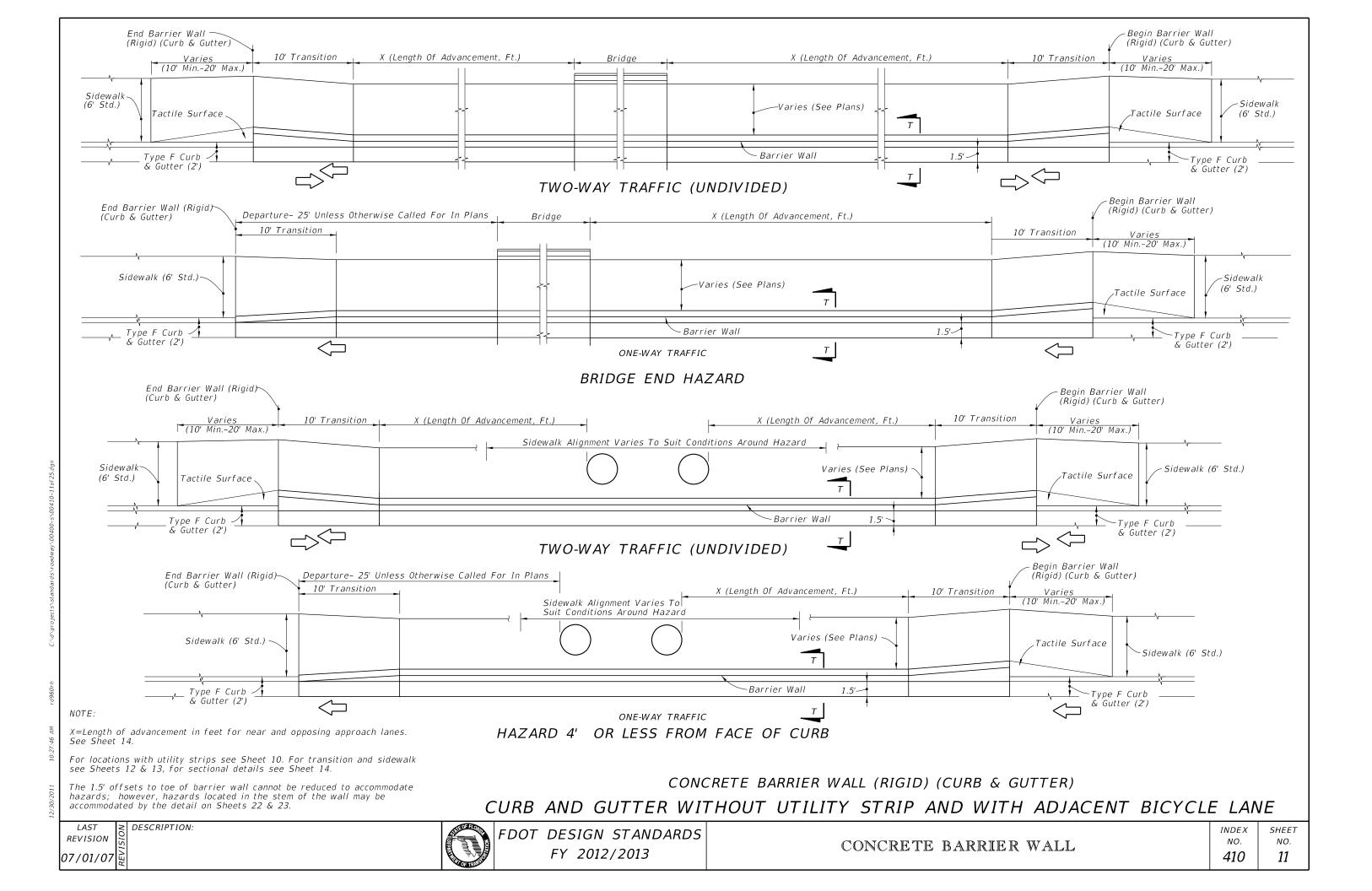


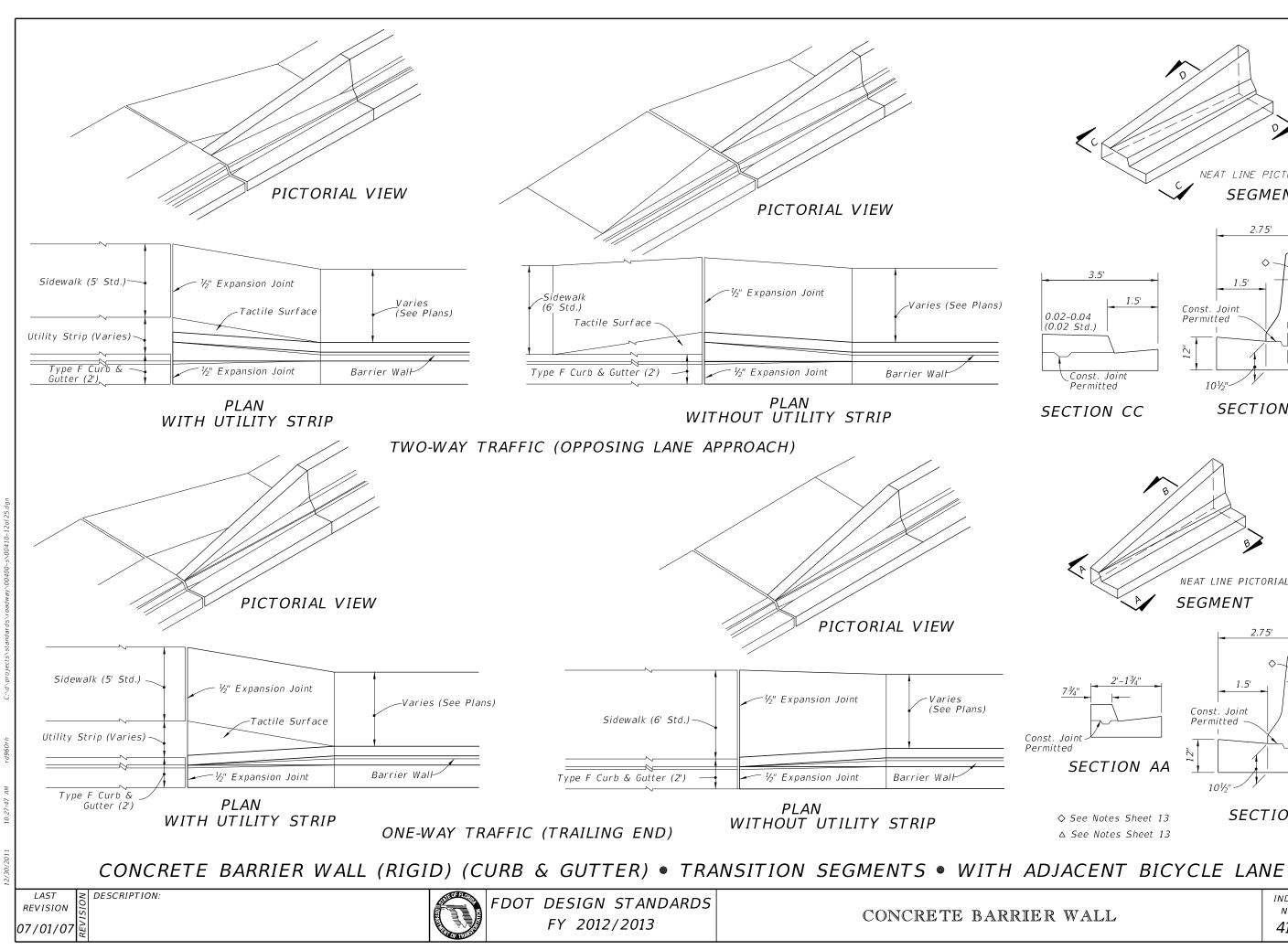
BENDING DIAGRAMS

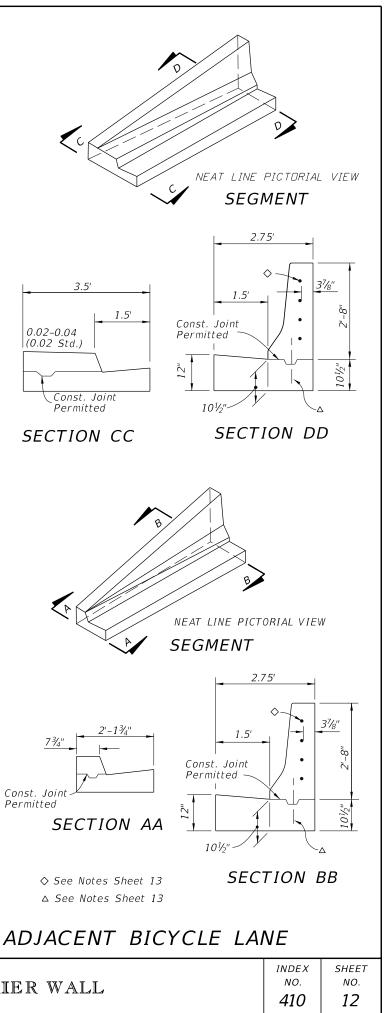


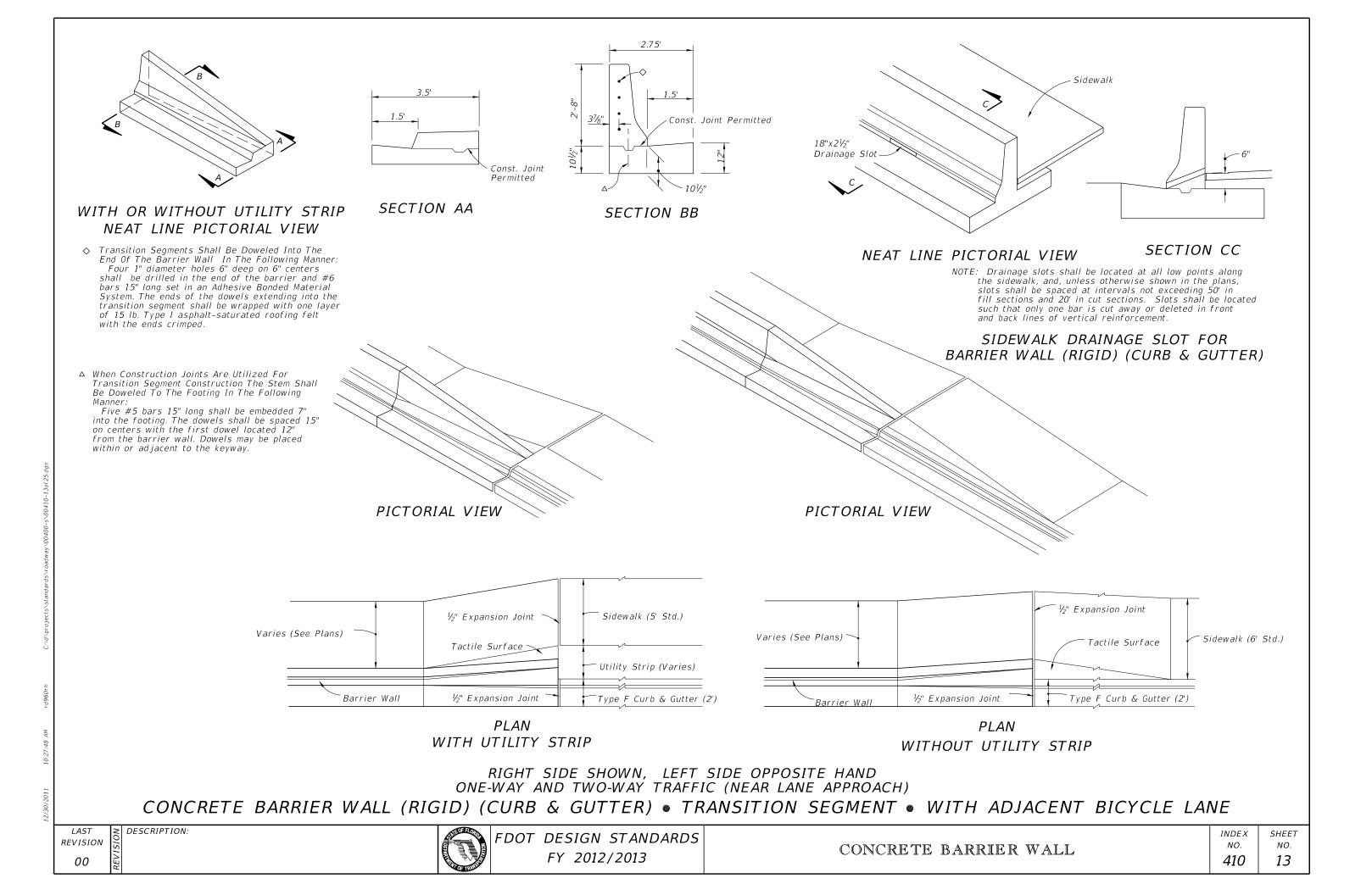


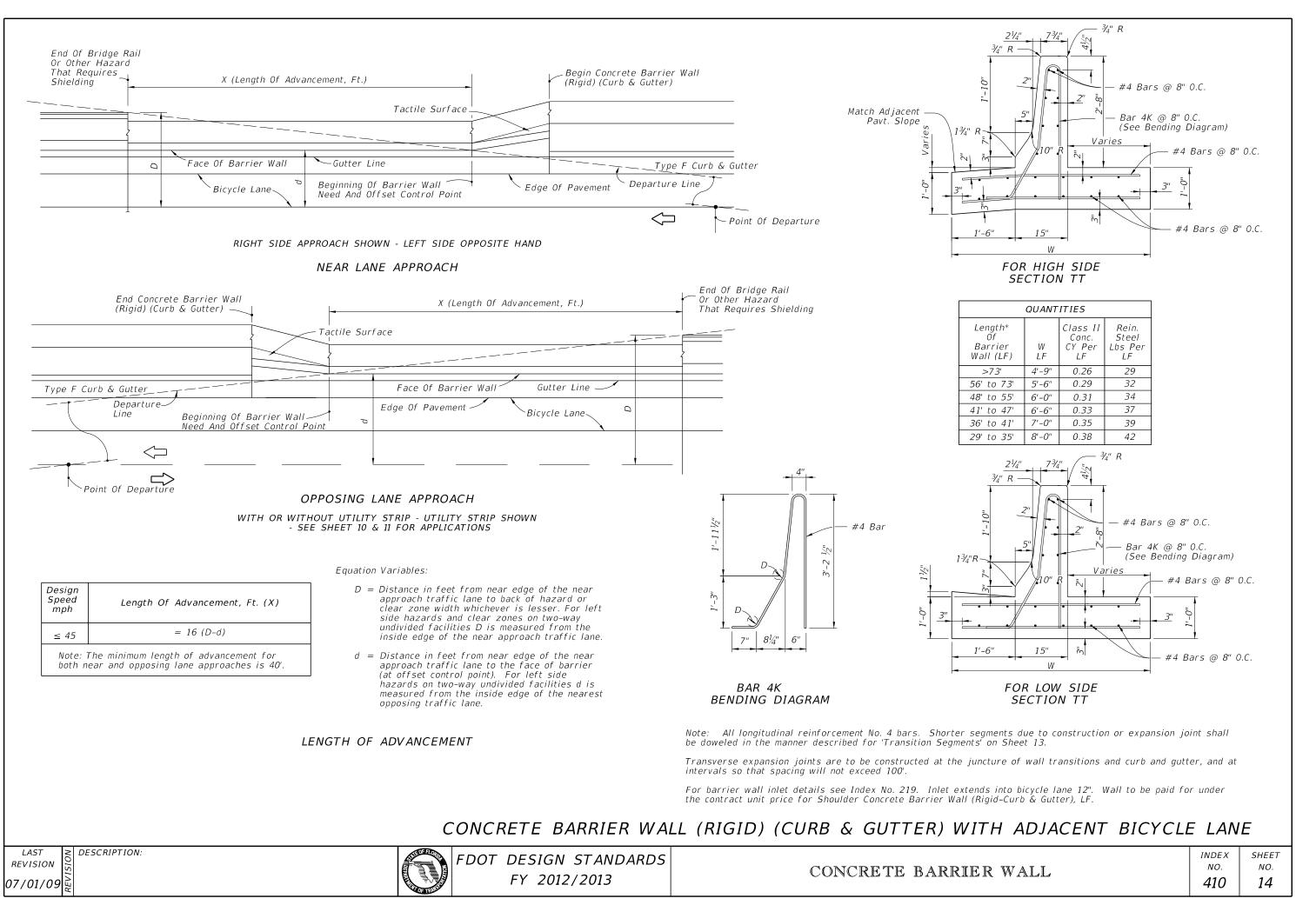
Wall Gutter)			
k (5' Std.)			
trip (Varies) NOTE:			
b & Gutter (2') See Sheet 14.			
For locations w see Sheet 11. For transition o see Sheets 12 Gutter) sectional detail	and sidewalk & 13 and for	deta	
(5' Std.) (5' St	reduced to azards; how in the stem commodated b	ever, of t	he
p (Varies)			
b & Gutter (2')			
on Begin Barrier Wall (Rigid) (Curb & Gutter)			
Sidewalk (5' St	d.)		
	_		
Utility Strip (Va	ries)		
	_		
Type F Curb & Gu	tter (2')		
, , , , , , , , , , , , , , , , , , ,			
Begin Barrier Wall (Rigid) (Curb & Gutte	r)		
Sidewalk (5' St	d.)		
Utility Strip (V			
Type F Curb & C	Gutter (2')		
H ADJACENT BICYCL	E LANE		
ER WALL	INC N 41	Э.	sheet NO. <b>10</b>

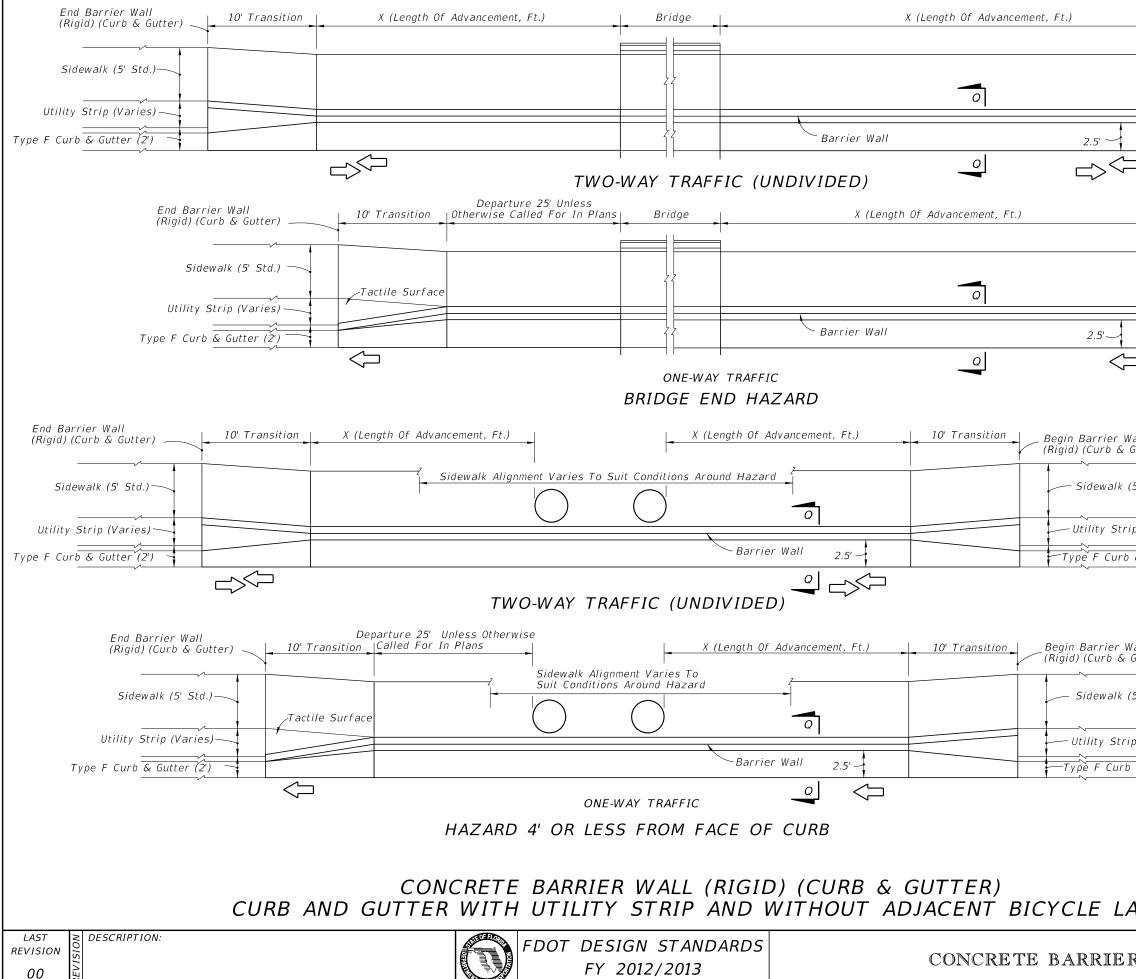




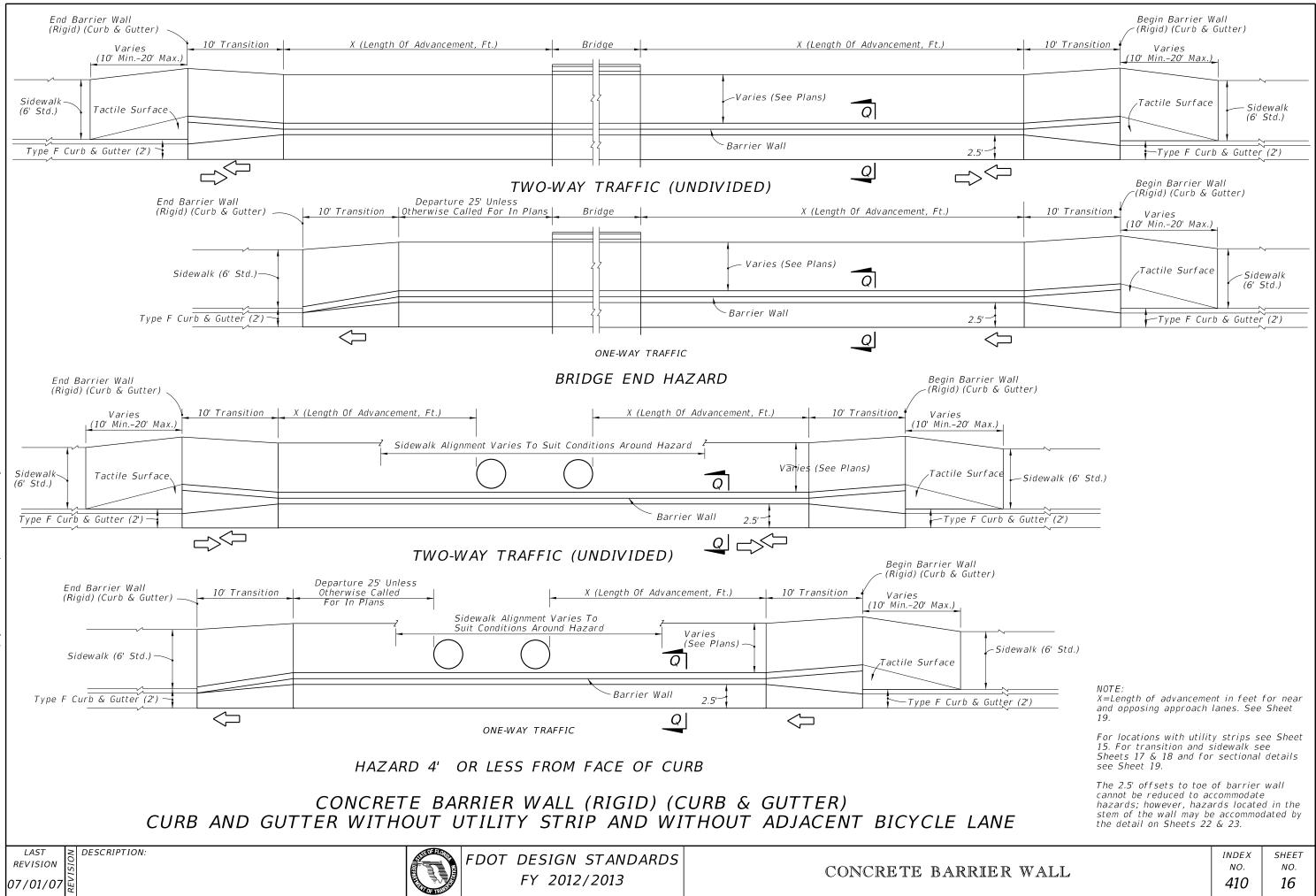


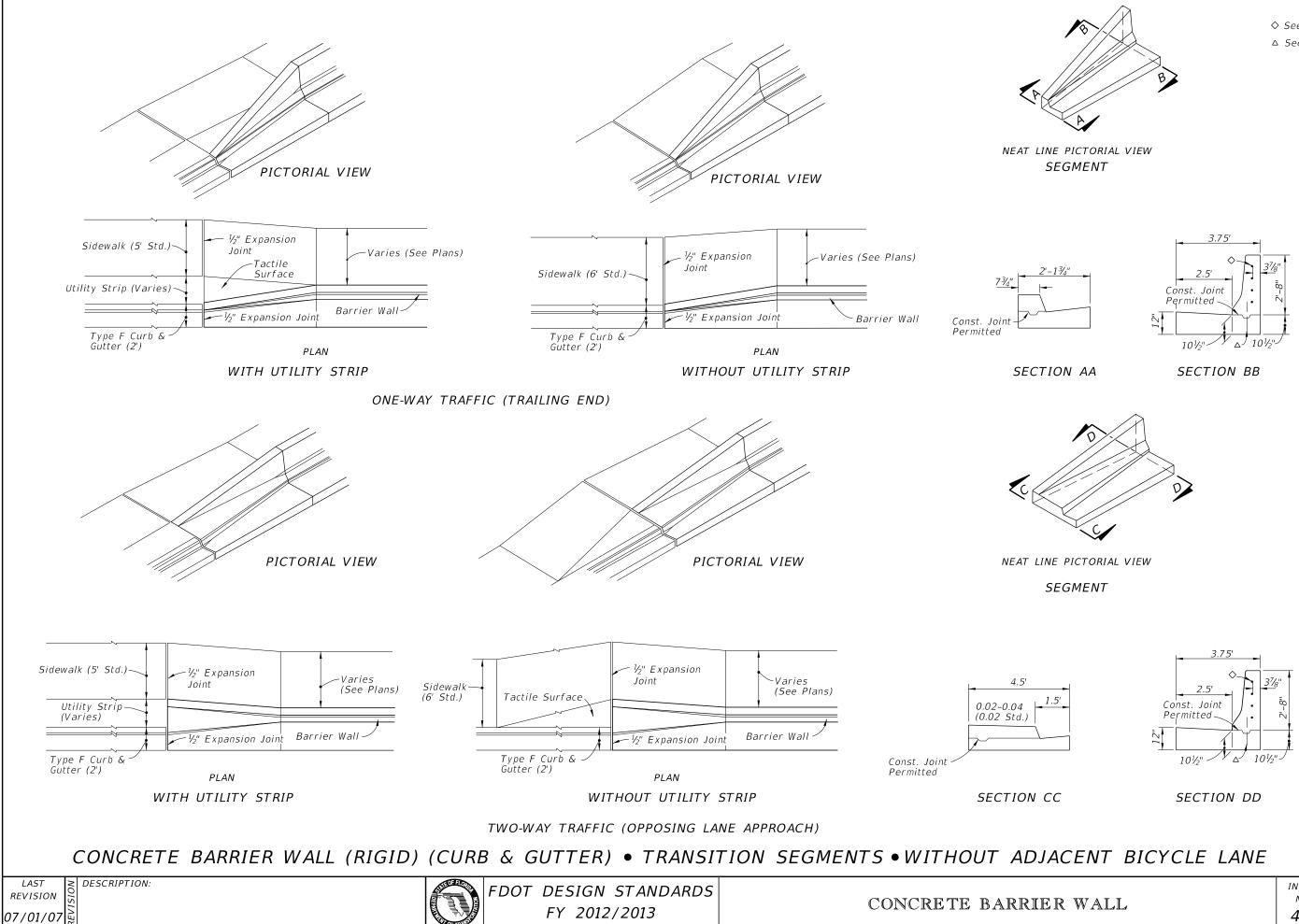






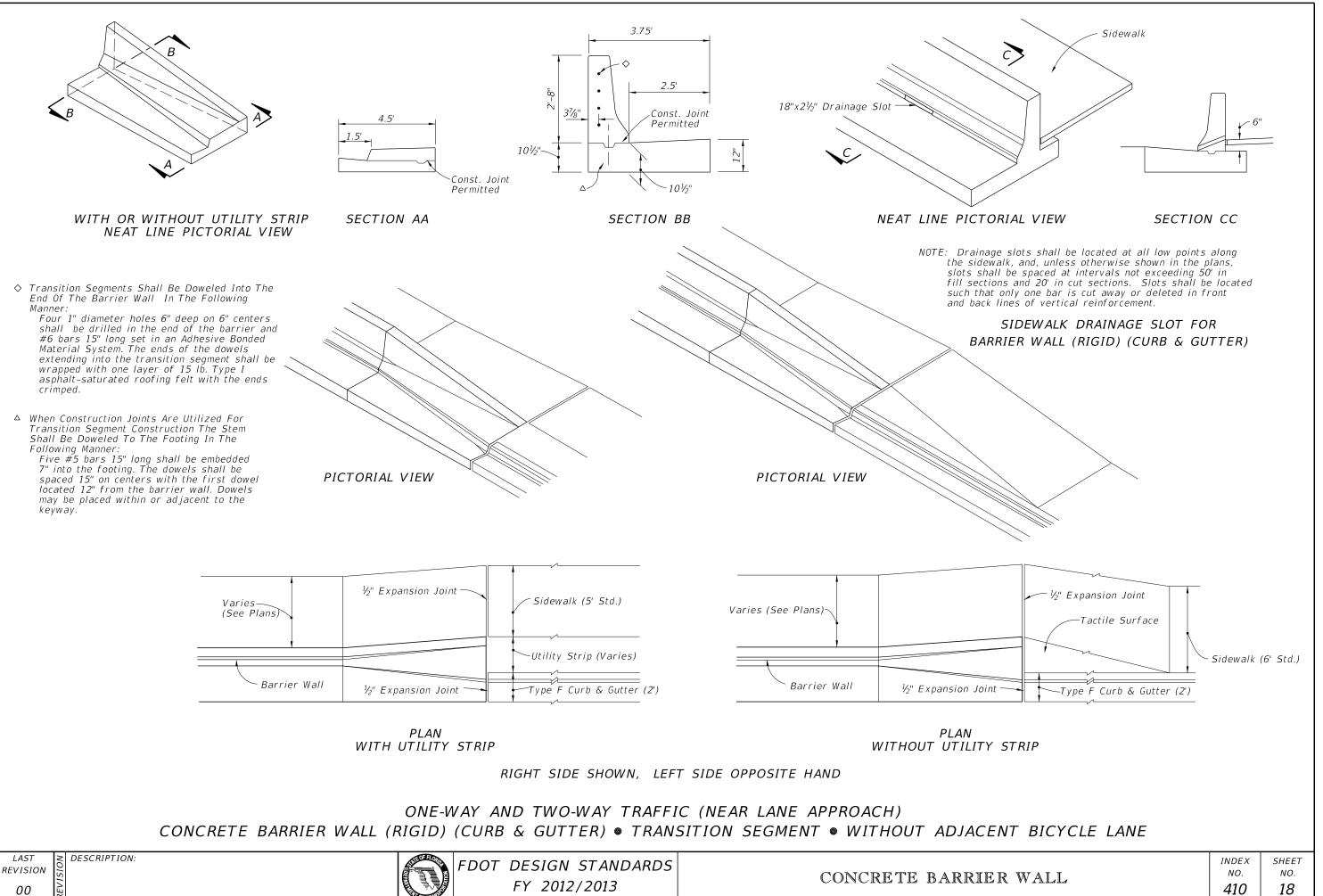
1	10	' Transition I	Begin Barrier	Wall	
-	4		(Rigid) (Curb &		
			<u>↓</u> ~		
			Sidewalk	(5' Std.)	
			<u> </u>		
			Utility St	trip (Varie	s)
			Type F Cur		
<u> </u>					. ,
I					
	10'	Transition	Begin Barrier N	Nall	
			Rigid) (Curb &	Gutter)	
			Gistanath		
			Sidewalk	(5 Sta.)	
			~		
			Utility St	rip (Varies	5)
			Type F Cur	b & Gutter	- (2')
<u> </u>			L		
1'					
Wall					
Gutte	er)				
151 6					
(5' S	ta.)				
rip (V	aries)				
•b & G	utter (	2')			
Wall & Gutte	er)				
(5' S	td.)				
rip (V	aries)				
rb&G	Gutter (	2')			
		NOTE: X=Length of adv	ancement in fe	et for nea	nr and
		opposing approad	ch lanes. See	Sheet 19.	
		For locations wit For transition ar and for sectional	d sidewalk se	e Sheets 1	7 & 18
		The 2.5' offsets	to toe of barr	ier wall ca	annot be
<b>.</b>	_	reduced to accon hazards located	in the stem of	the wall	may be
.AN	E	accommodated by	the details o	n Sheets 2	2 & 23.
				INDEX	SHEET
CR 1	VAL			NO.	NO.
апс Г	, x x 11-			410	15



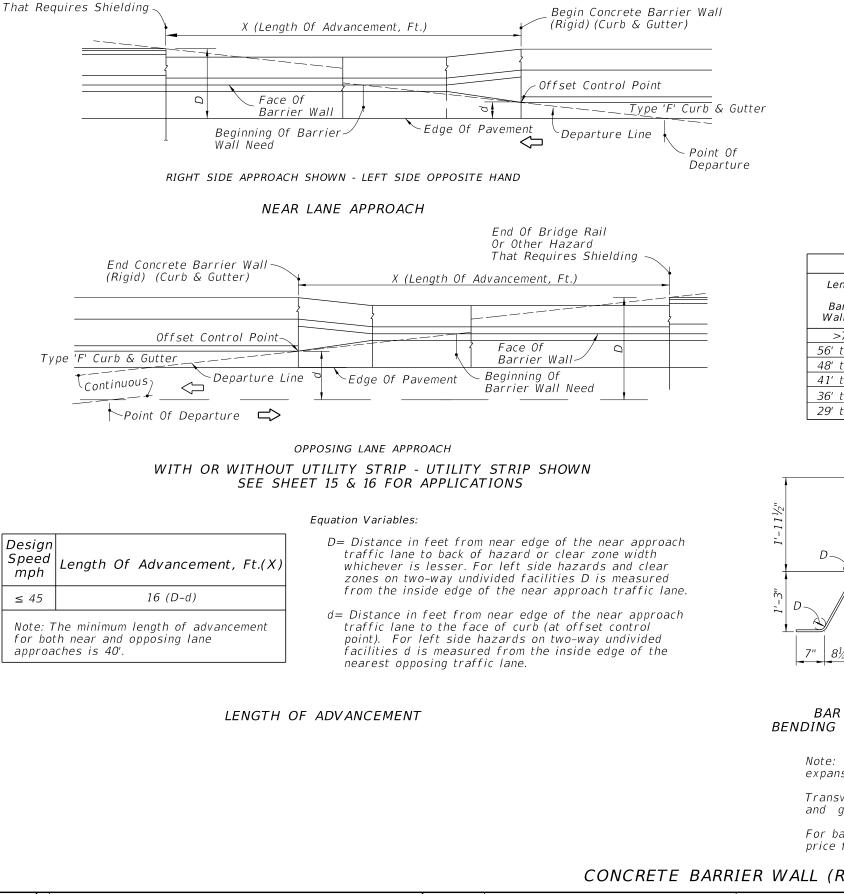


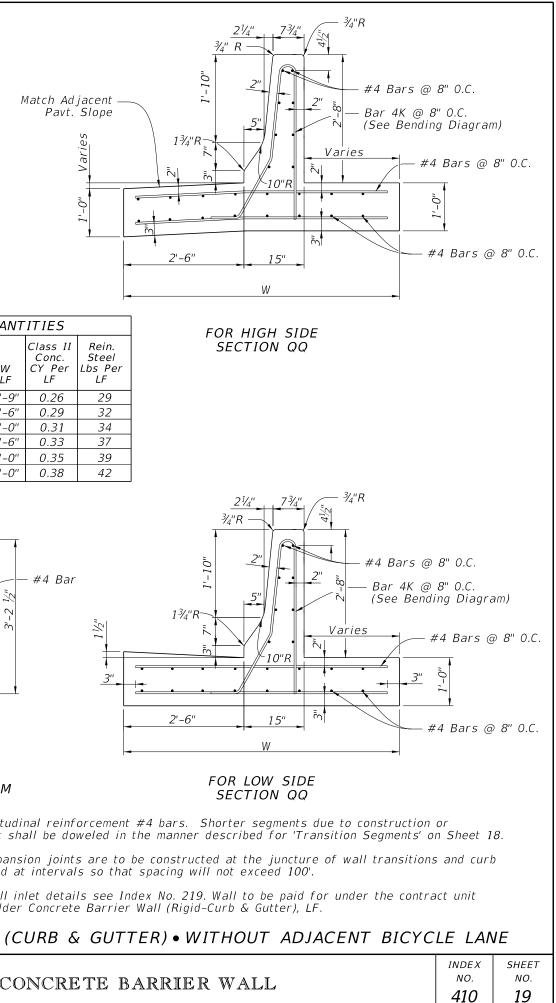
♦ See Sheet 19  $\triangle$  See Sheet 19

	INDEX	SHEET
CR WALL	NO.	NO.
	410	17

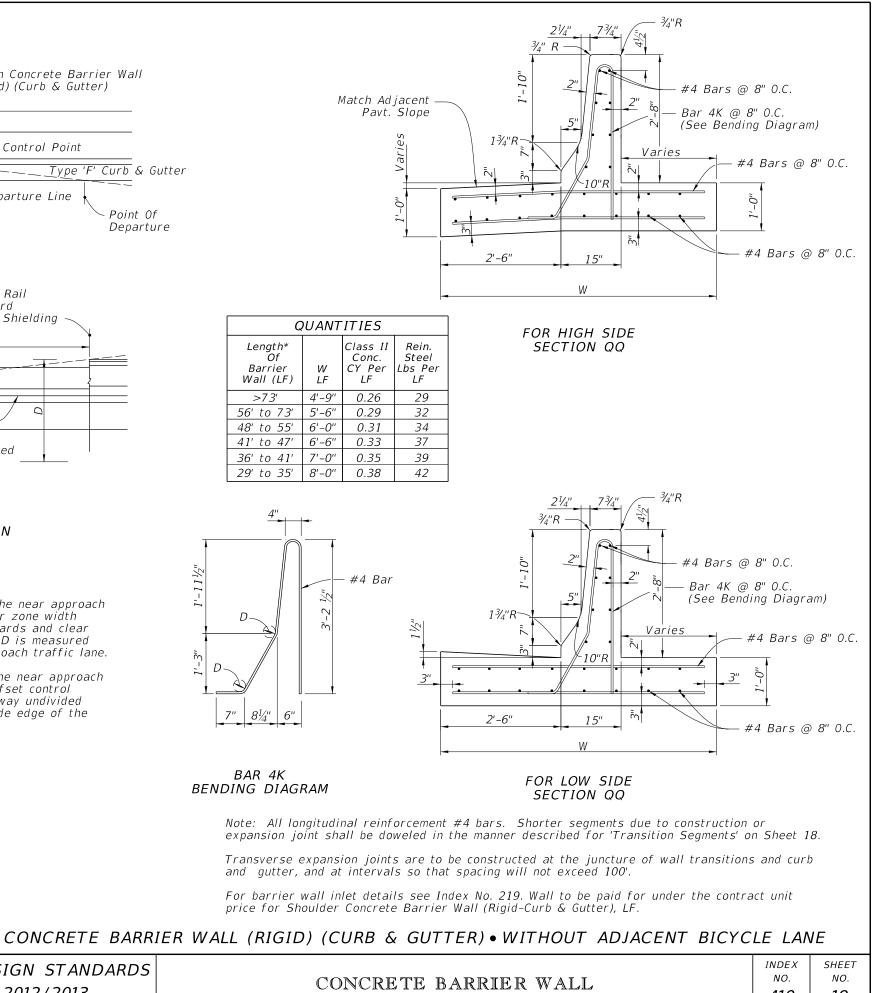


# End Of Bridge Rail Or Other Hazard





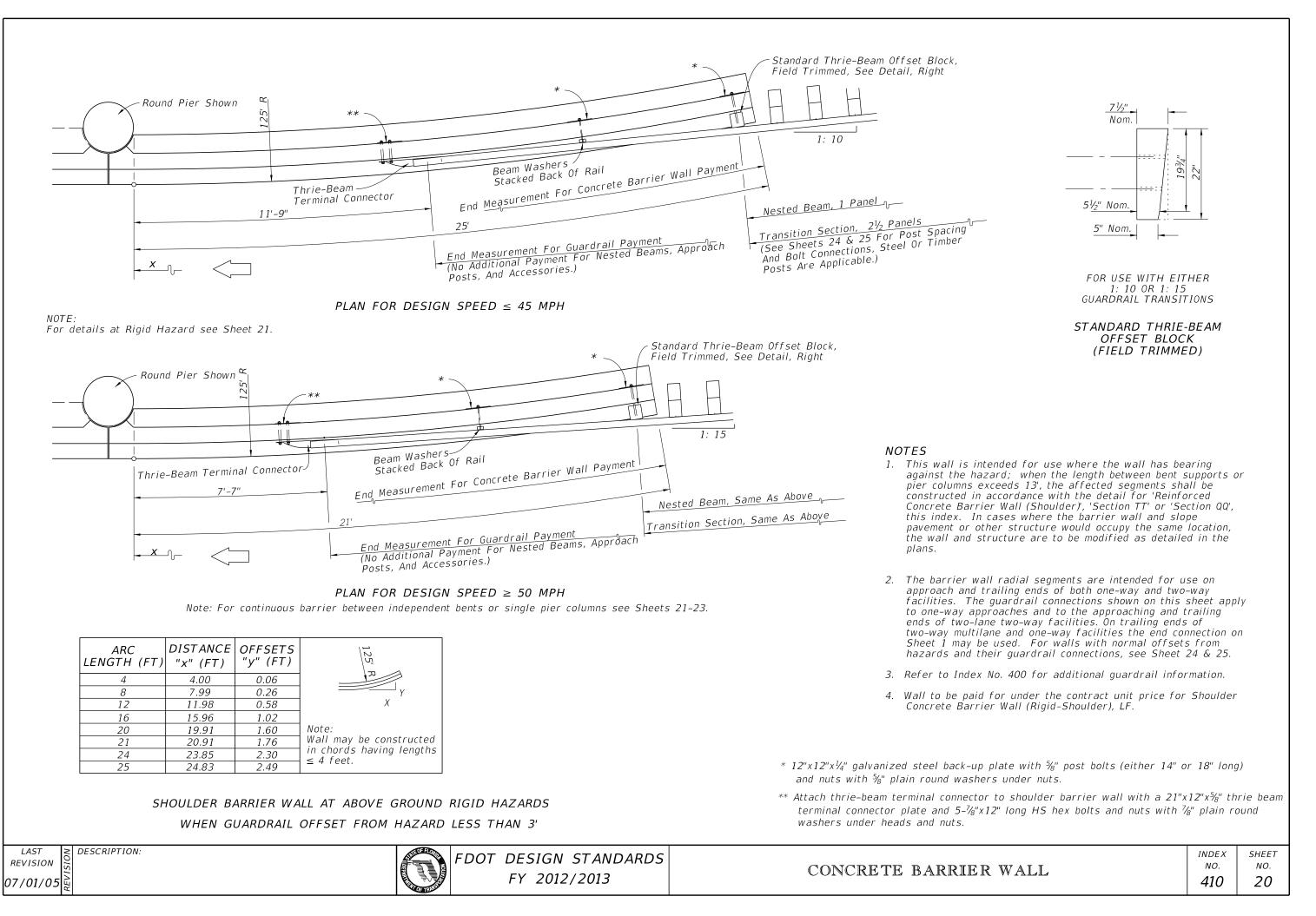
QUANTITIES								
Length* Of Barrier Wall (LF)	W LF	Class II Conc. CY Per LF	Rein. Steel Lbs Per LF					
>73'	4'-9"	0.26	29					
56' to 73'	5'-6"	0.29	32					
48' to 55'	6'-0''	0.31	34					
41' to 47'	6'-6"	0.33	37					
36' to 41'	7'-0''	0.35	39					
29' to 35'	8'-0"	0.38	42					



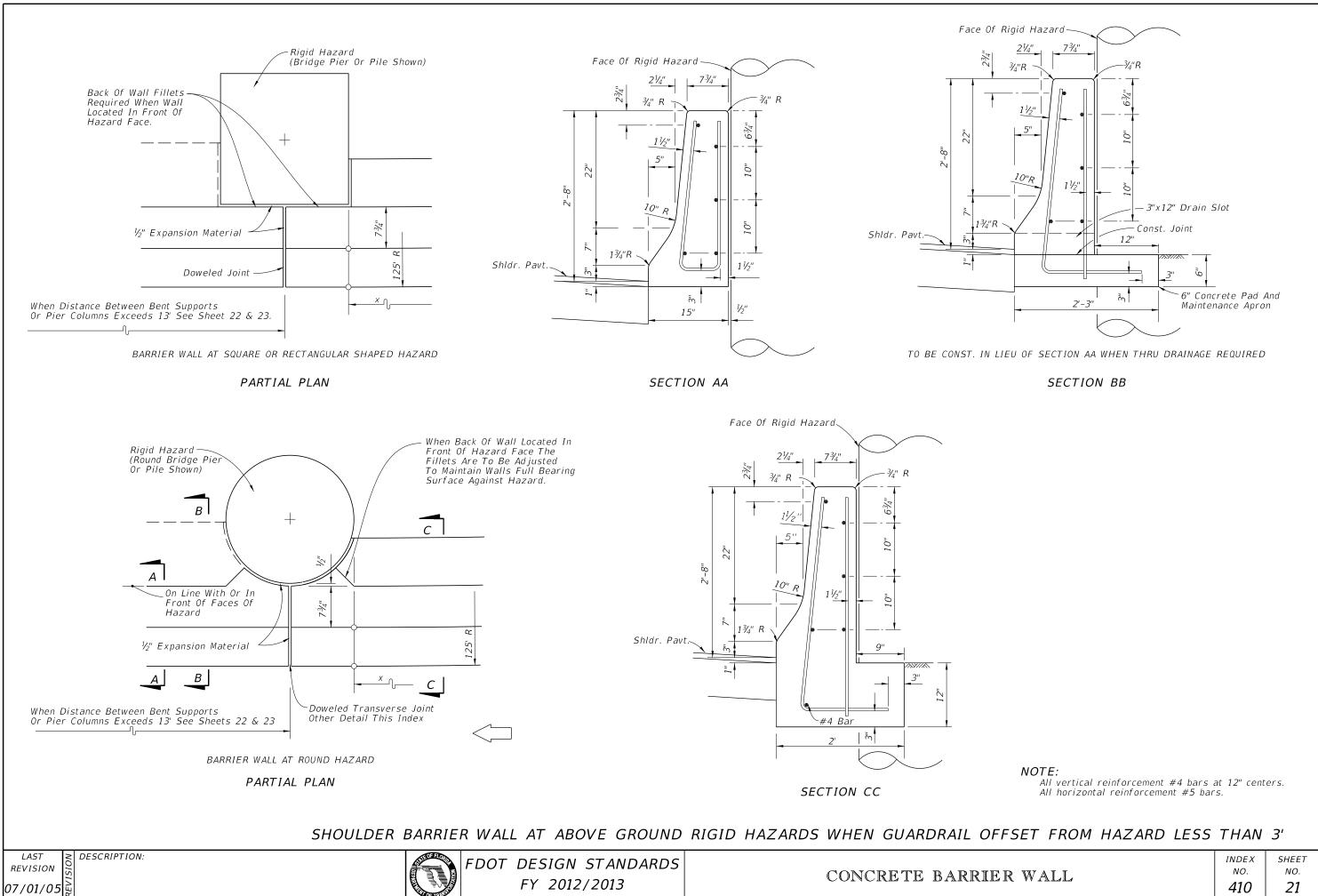
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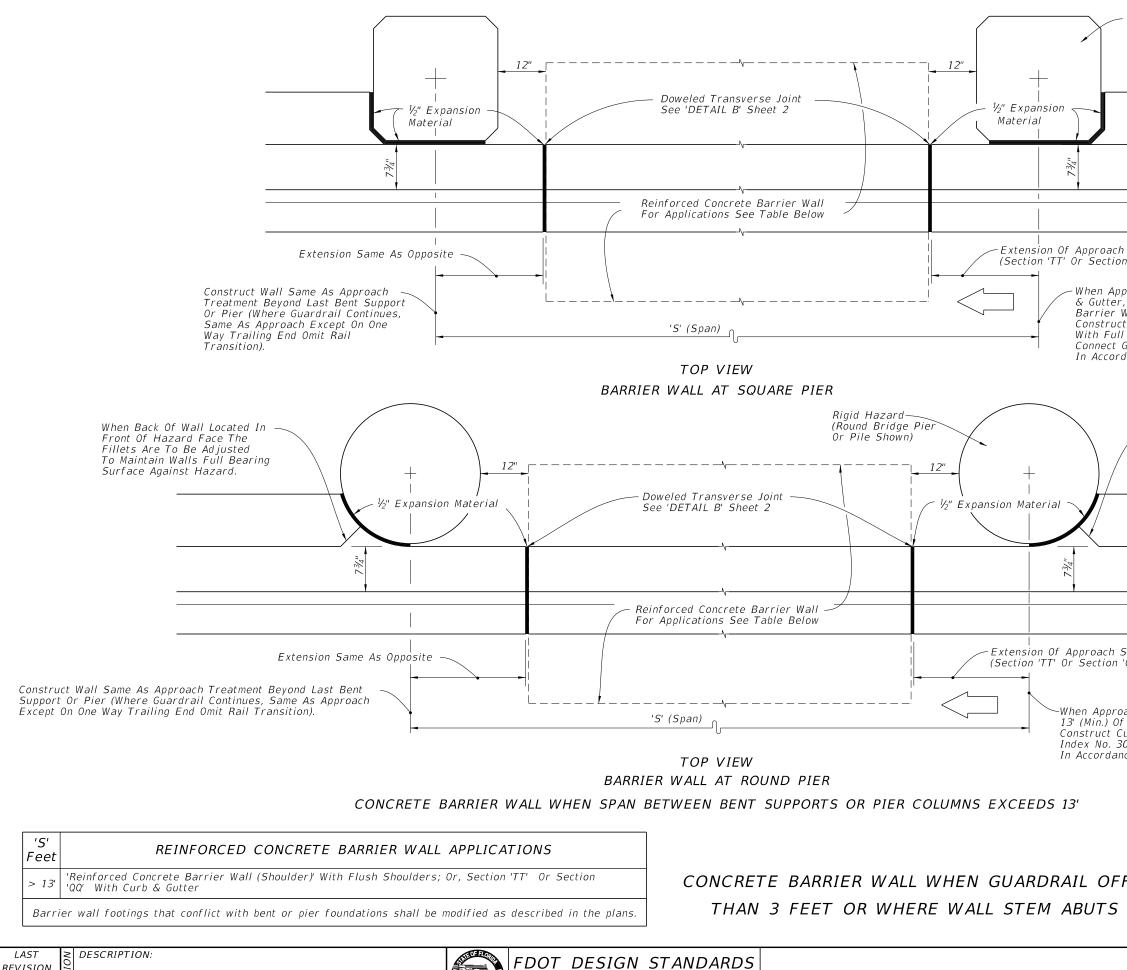


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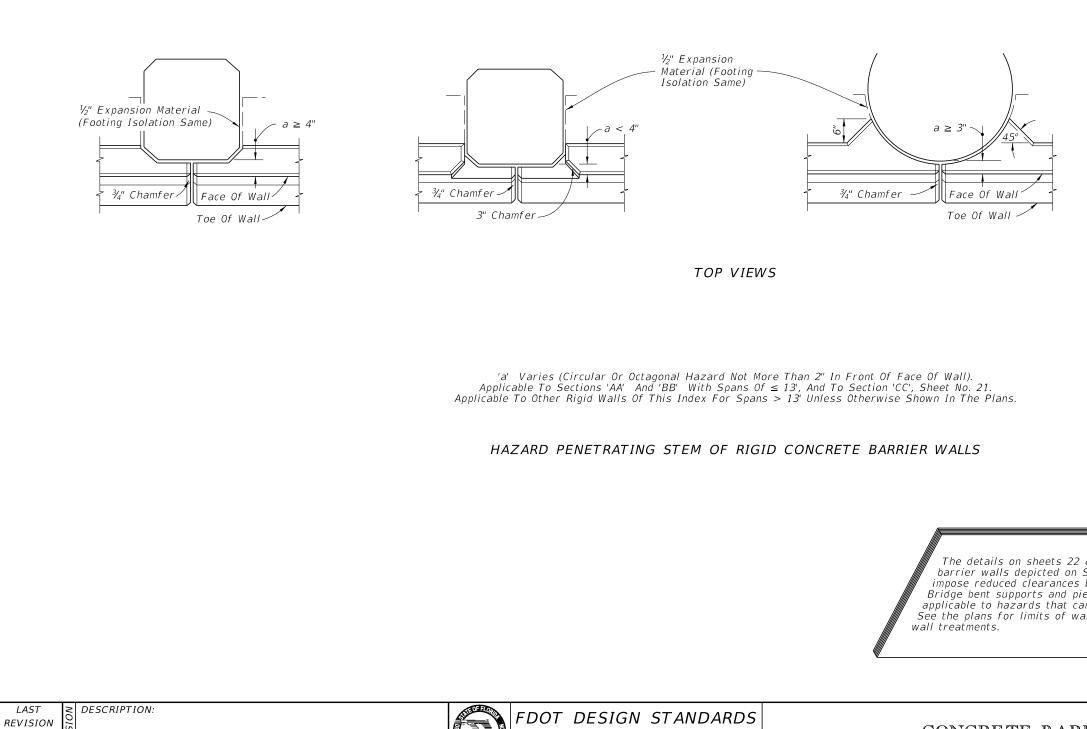




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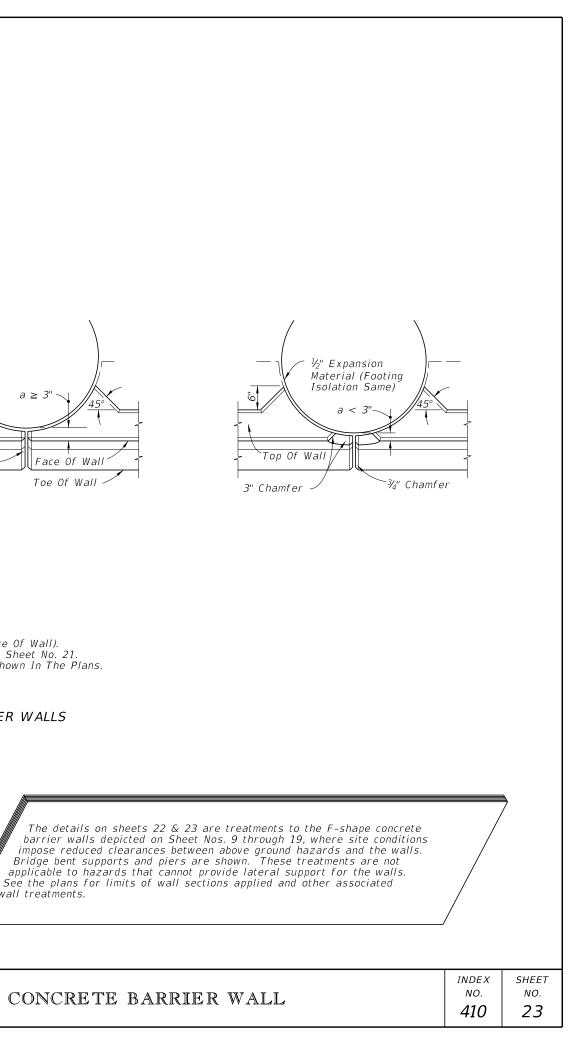
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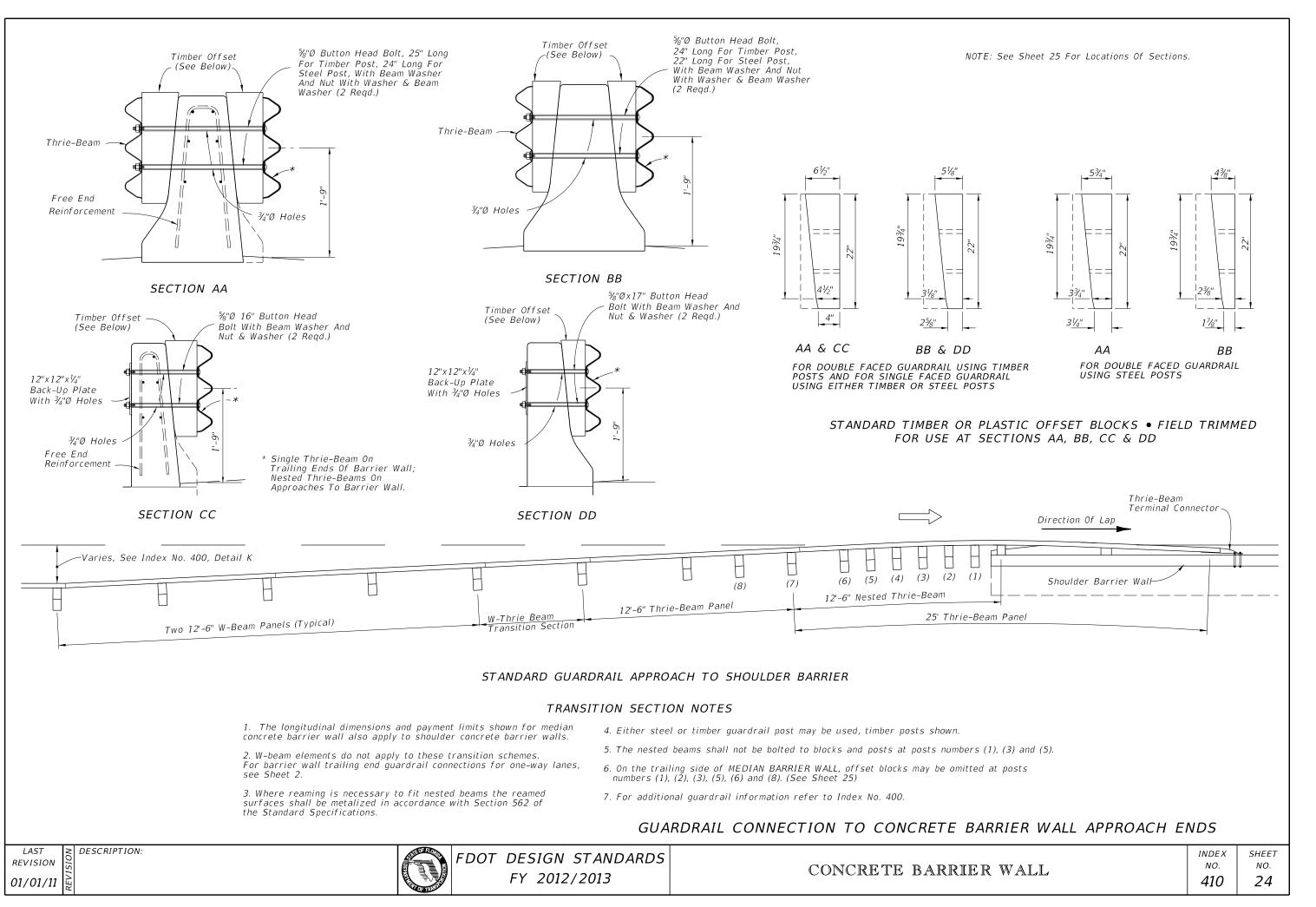
Rigid Hazard (Square Bridge Pier Or Pile Shown)	
<sup>1</sup> ⁄ <sub>2</sub> " Expansion Material	
Extension Of Approach Section 'AA', 'BB' Or 'CC' (Section (Section 'TT' Or Section 'QQ' With Curb & Gutter Approac	n 'CC' Shown) ch)
When Approach Shielding Is Guardrail And & Gutter, Construct 13' (Min.) Of Concrete Barrier Wall, Section 'TT' Or Section 'QQ'; Construct Curb & Gutter Flare At End Of With Full Height Curb, Index No. 300; And Connect Guardrail To Wall With Transition In Accordance With Sheet Nos. 24 & 25.	Wall
When Back Of Wall Locate Front Of Hazard Face Th Fillets Are To Be Adjuste To Maintain Walls Full Be Surface Against Hazard.	ne ed
Extension Of Approach Section 'AA', 'BB' Or 'CC' (Section 'G (Section 'TT' Or Section 'QQ' With Curb & Gutter Approach, When Approach Shielding Is Guardrail And Co 13' (Min.) Of Concrete Barrier Wall, Section ' Construct Curb & Gutter Flare At End Of Wa Index No. 300; And, Connect Guardrail To Wa In Accordance With Sheet Nos. 24 & 25.	) urb & Gutter, Construct TT' Or Section 'QQ'; Il With Full Height Curb,
DLUMNS EXCEEDS 13'	
WHEN GUARDRAIL OFFSET FROM BENT OI E WALL STEM ABUTS SUPPORTS OR PIER	
CONCRETE BARRIER WALL	INDEX SHEET NO. NO. 410 22

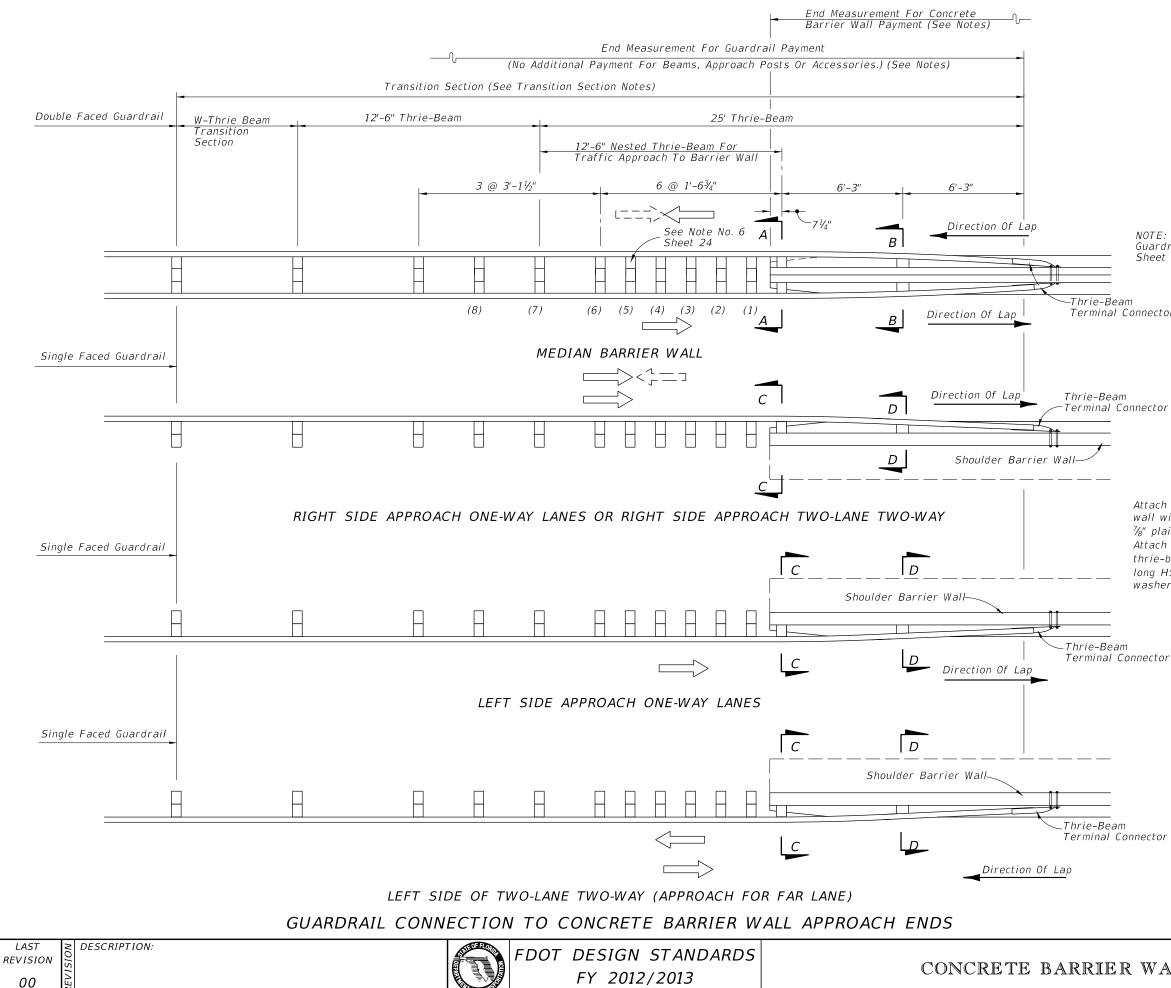


07/01/07

CONCRETE BARRIER WALL







NOTE: For Section AA, BB, CC and DD Guardrail and Offset Block Views, See Sheet 24

Terminal Connector

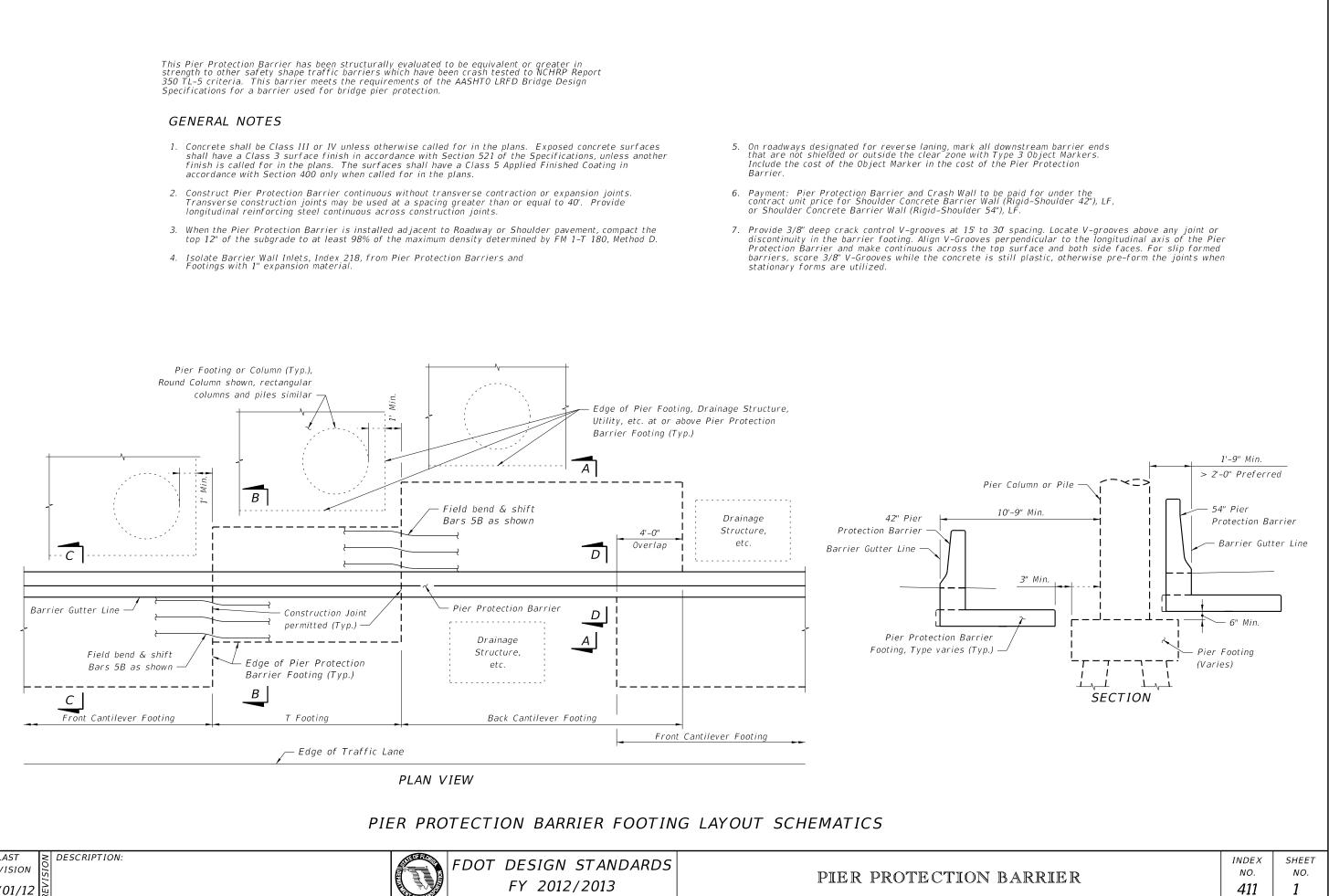
Attach thrie-beam terminal connector to median barrier wall with 5- $\frac{7}{8}$ "x15" long HS hex bolts and nuts with  $7_{\!\!8}^{\prime\prime\prime}$  plain round washers under heads and nuts. Attach to shoulder barrier wall with a 21"x12"x $\frac{5}{8}$ " thrie-beam terminal connector plate and 5-7/8"x12" long HS hex bolts and nuts with 7/8" plain round washers under heads and nuts.

-		INDEX NO.	SHEET NO.
R	WALL	410	25

350 TL-5 criteria. This barrier meets the requirements of the AASHTO LRFD Bridge Design Specifications for a barrier used for bridge pier protection.

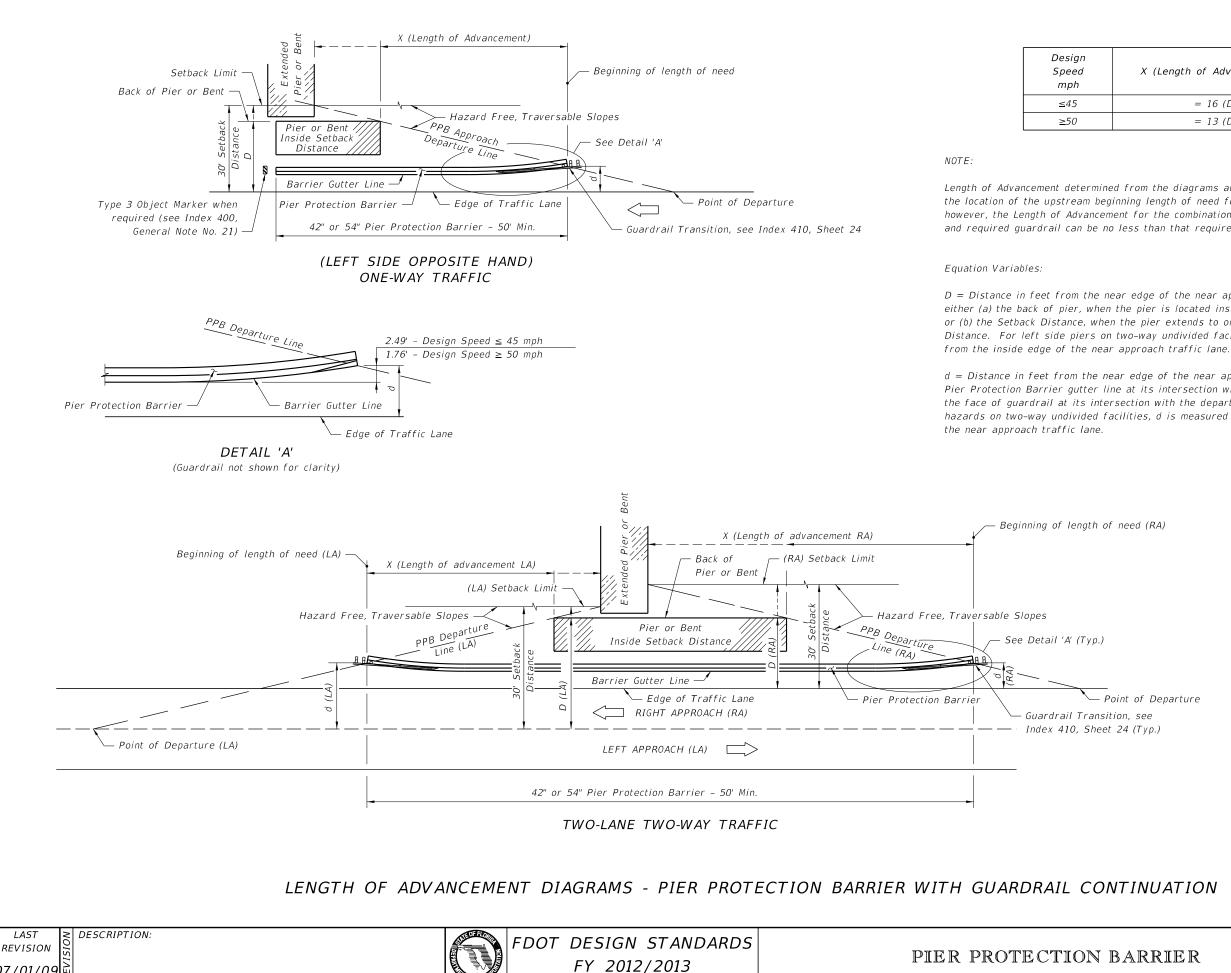
- shall have a Class 3 surface finish in accordance with Section 521 of the Specifications, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finished Coating in accordance with Section 400 only when called for in the plans.
- Transverse construction joints may be used at a spacing greater than or equal to 40'. Provide

- Barrier
- stationary forms are utilized.



LAST REVISION 01/01/12





07/01/09

PIER PROTECTION B

X (Length of Advancement)	Ft.
= 16 (D-d)	
= 13 (D-d)	

Length of Advancement determined from the diagrams and equations shown establishes the location of the upstream beginning length of need for a Pier Protection Barrier, however, the Length of Advancement for the combination of Pier Protection Barrier and required guardrail can be no less than that required by other details of Index 400.

D = Distance in feet from the near edge of the near approach traffic lane to either (a) the back of pier, when the pier is located inside the Setback Distance or (b) the Setback Distance, when the pier extends to or goes beyond the Setback Distance. For left side piers on two-way undivided facilities, D is measured

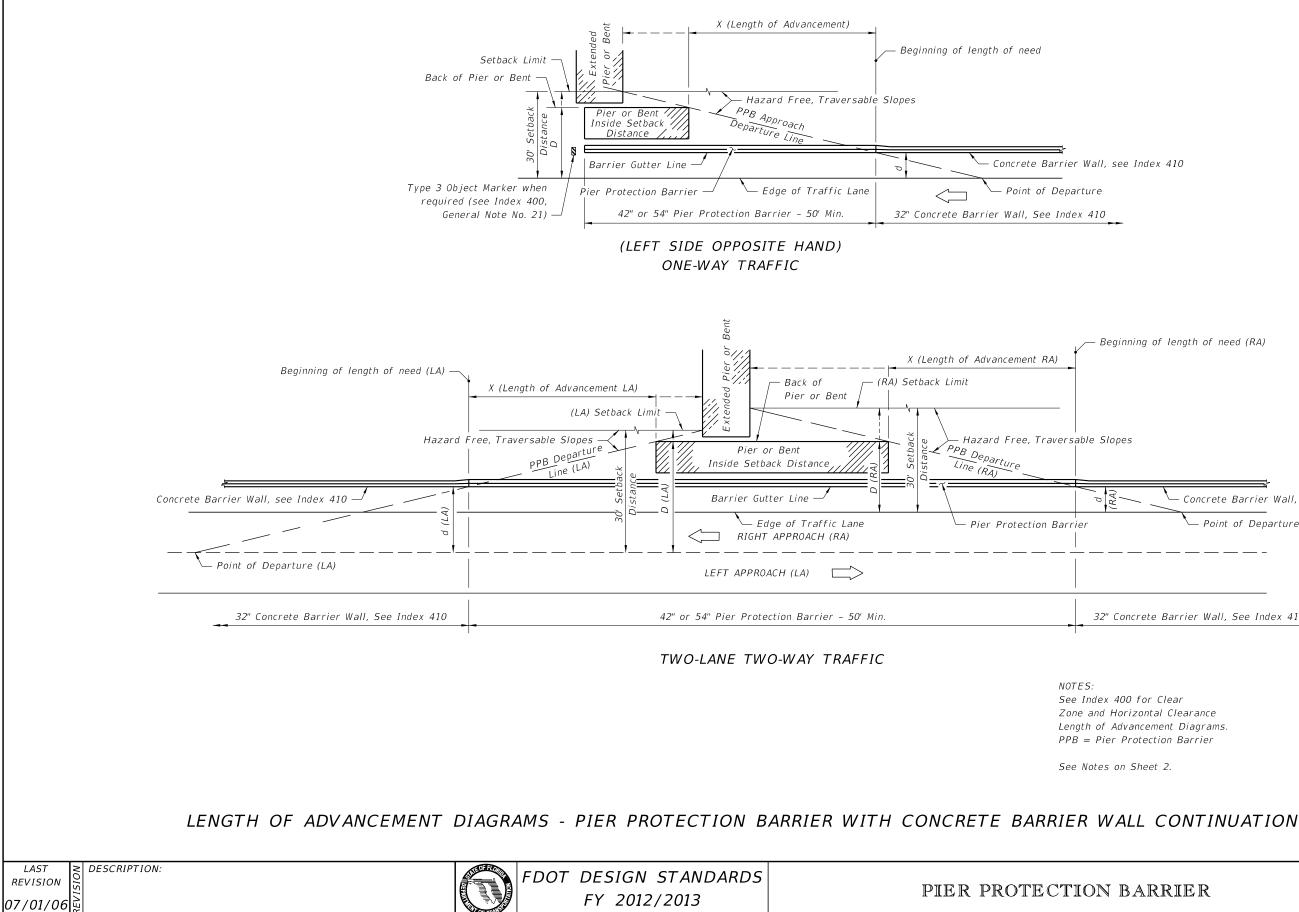
d = Distance in feet from the near edge of the near approach traffic lane to the Pier Protection Barrier gutter line at its intersection with the departure line or the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of

— Point of Departure

NOTE: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams.

PPB = Pier Protection Barrier

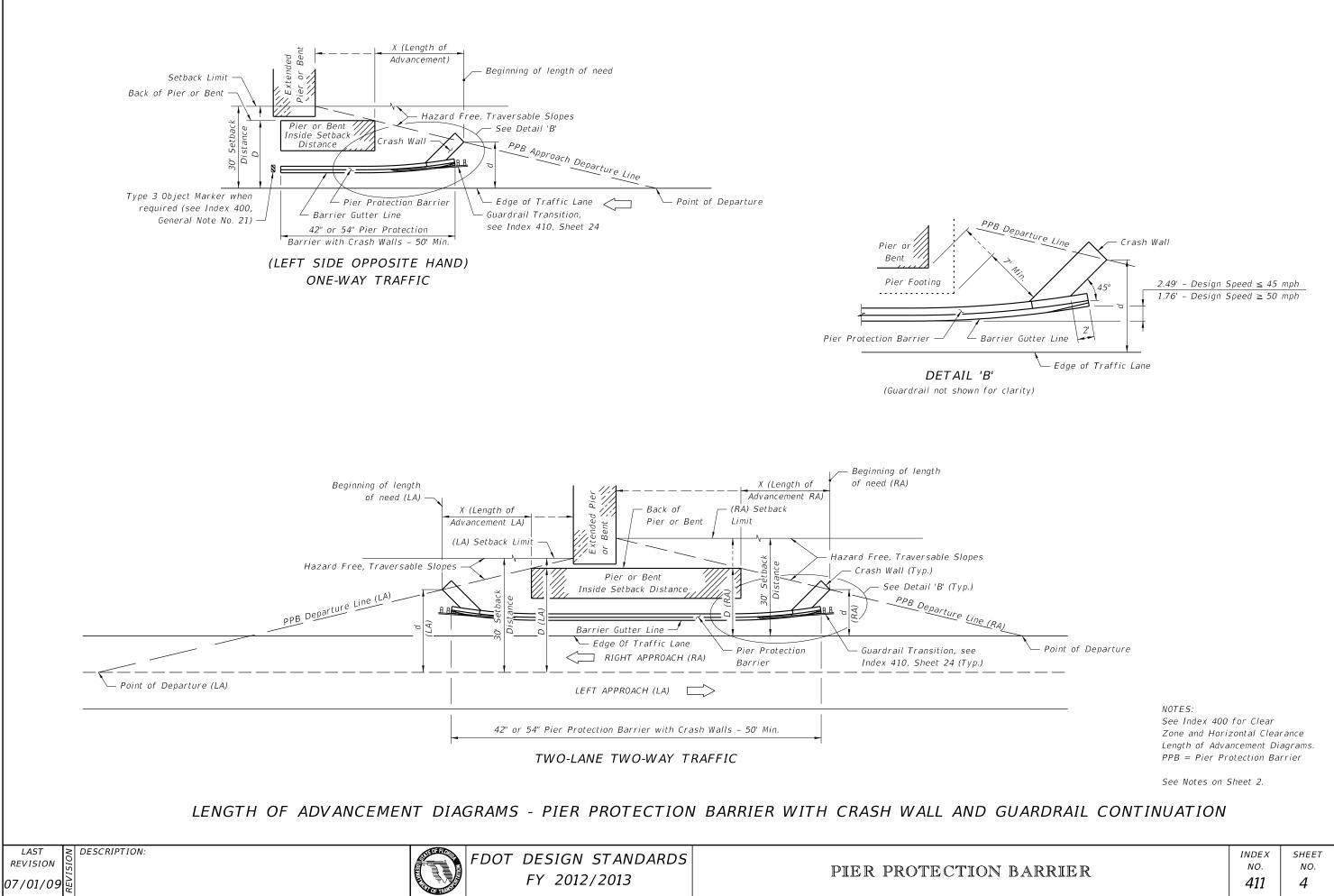
	INDEX	SHEET
BARRIER	NO.	NO.
	411	2

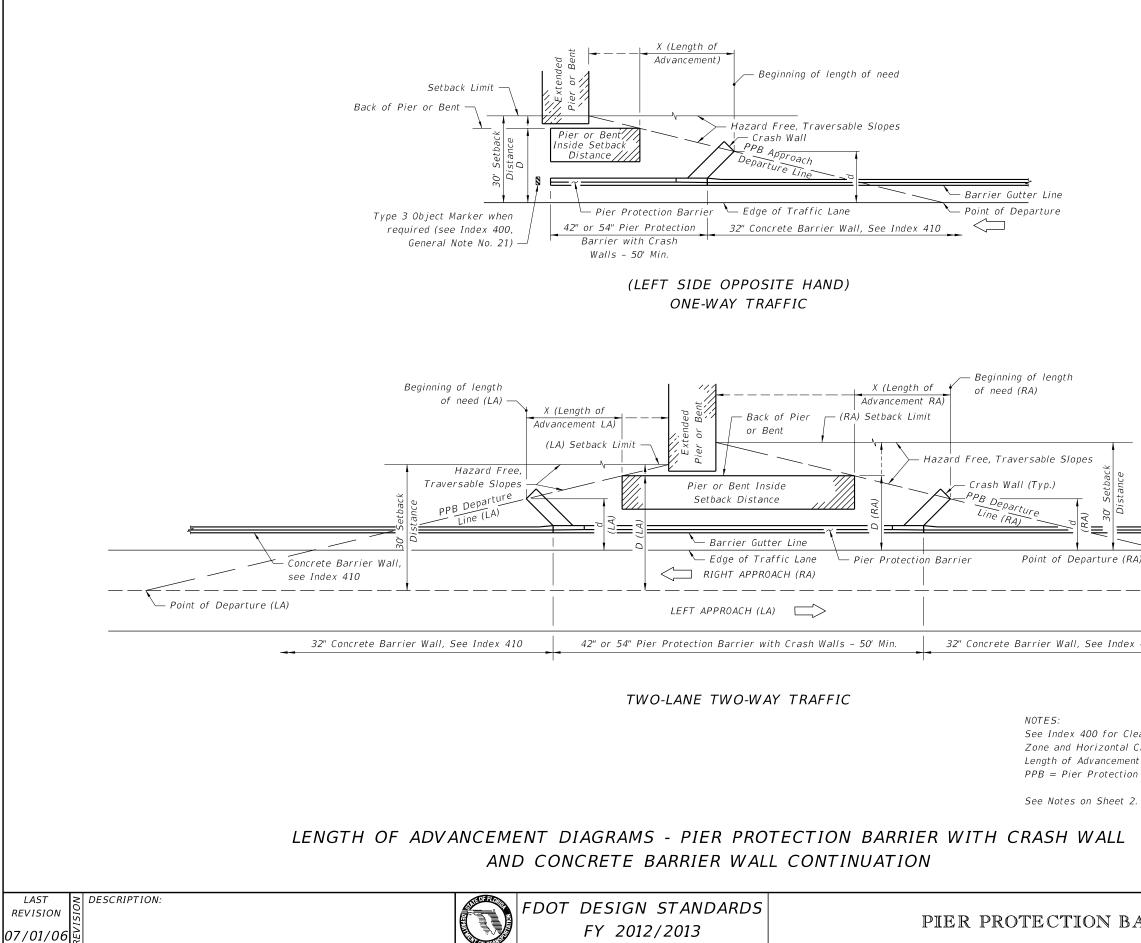


- Beginning of length of need (RA)

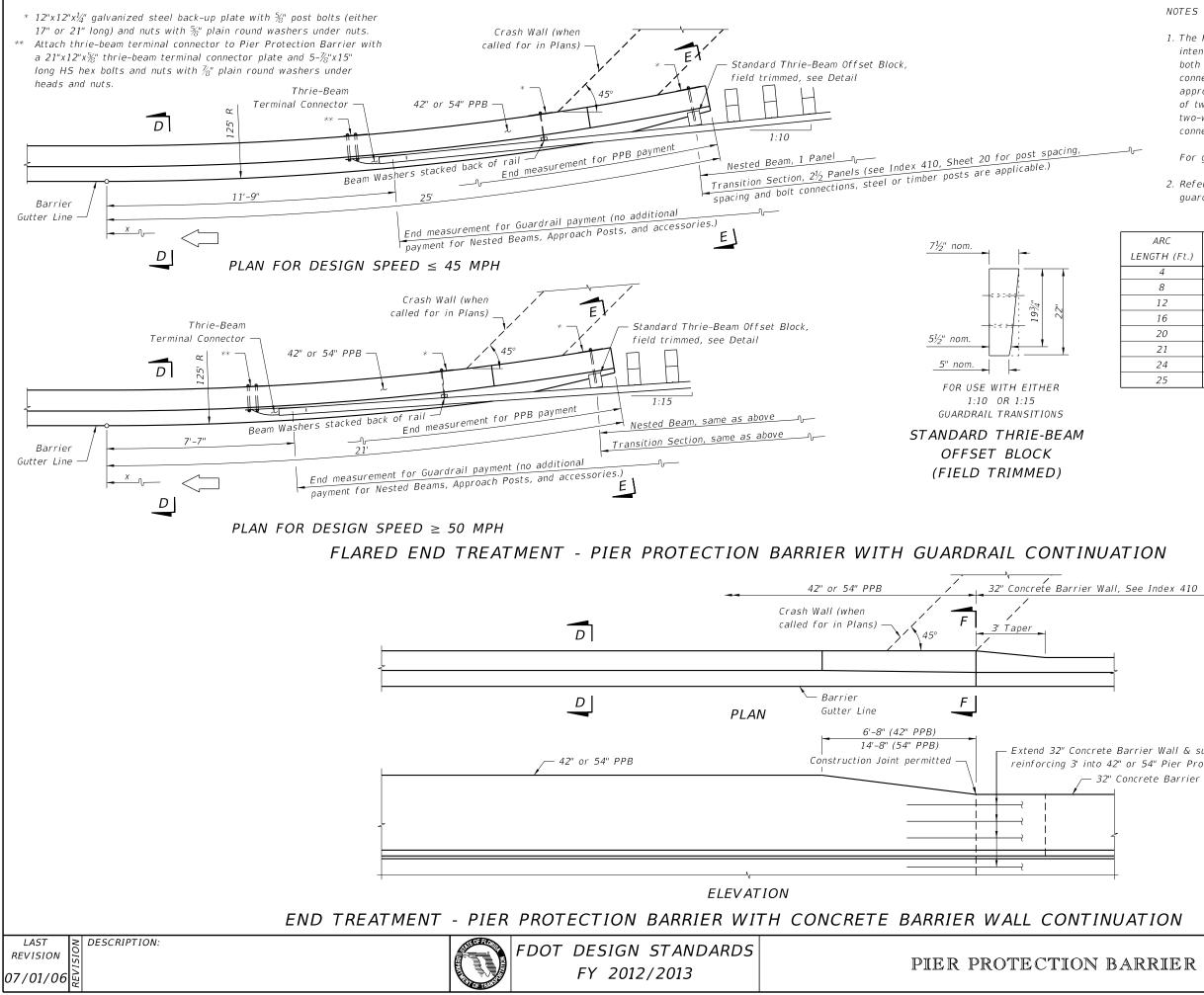
Concrete Barrier Wall, see Index 410
Point of Departure
ncrete Barrier Wall, See Index 410
0 for Clear rizontal Clearance
ancement Diagrams.
Protection Barrier
Sheet 2.

	INDEX	SHEET
BARRIER	NO.	NO.
	411	3





RA) — Concrete Barrier Wall, See Index 410		
ex 410		
Clear I Clearance ent Diagrams. ion Barrier 2.		
BARRIER	index no. <b>411</b>	sнеет NO. <b>5</b>



## NOTES

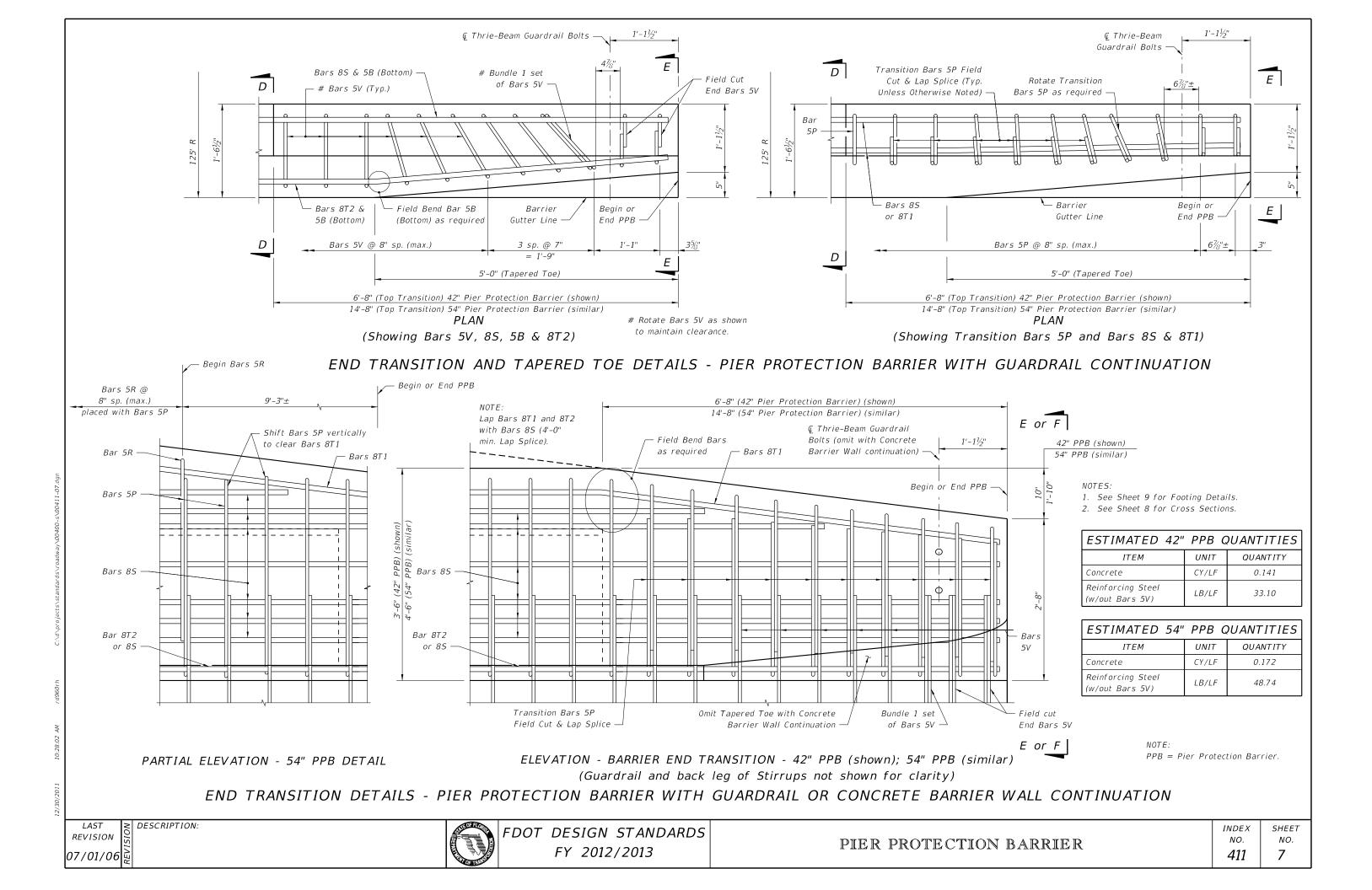
- 1. The Pier Protection Barrier radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane two-way facilities. On trailing ends of two-way multilane and one-way facilities the end connection on Index 410, Sheet 2 may be used.
- For guardrail connections, see Index 410, Sheet 20.
- 2. Refer to Index No. 400 Detail J for additional guardrail information.

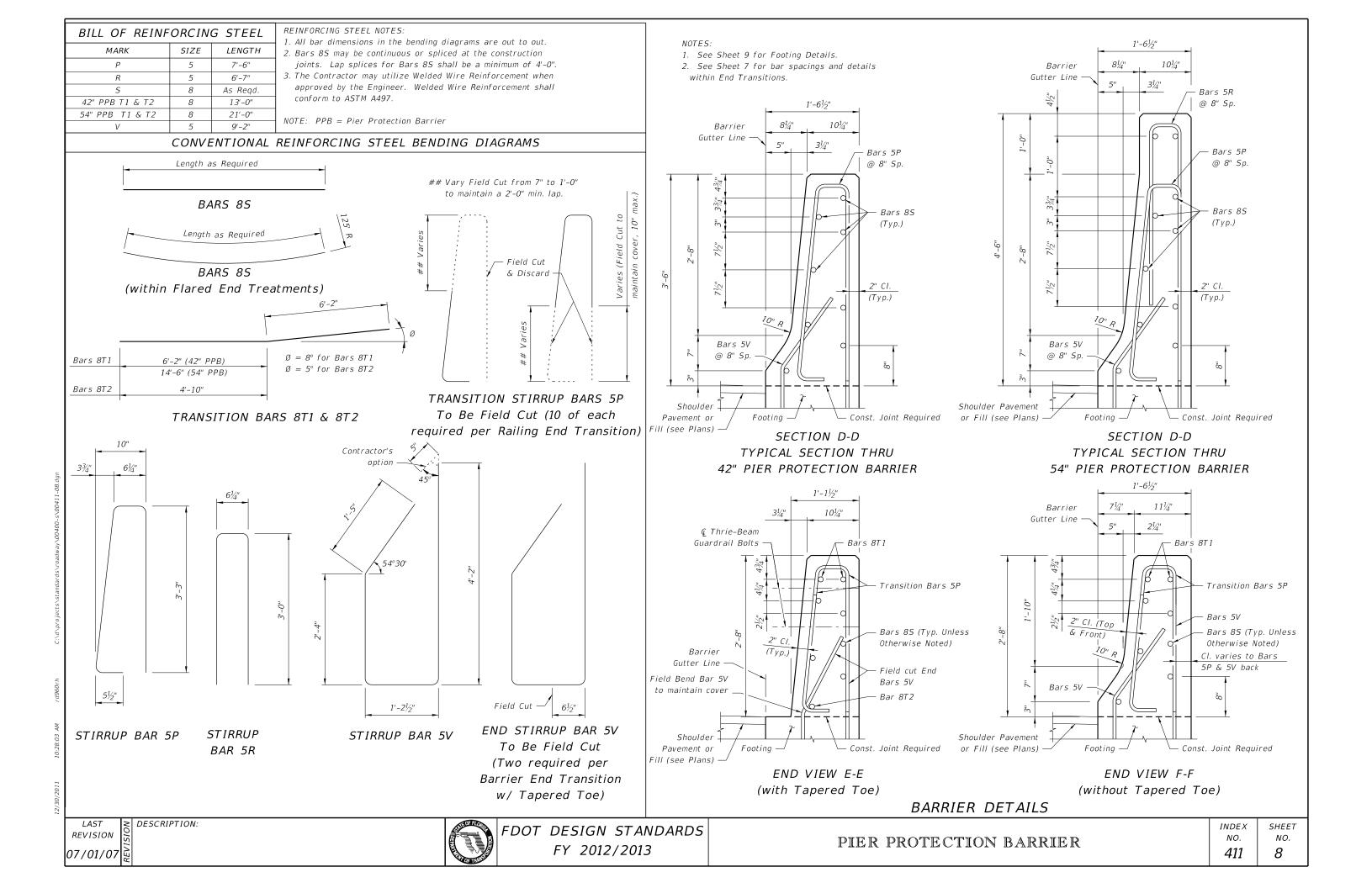
ARC	DISTANCE	OFFSETS "y"	
LENGTH (Ft.)	"x" (Ft.)	"y" (Ft.)	125
4	4.00	0.06	R
8	7.99	0.26	Y
12	11.98	0.58	X
16	15.96	1.02	Note:
20	19.91	1.60	Barrier may be
21	20.91	1.76	constructed in chords having
24	23.85	2.30	lengths ≤4 feet.
25	24.83	2.49	

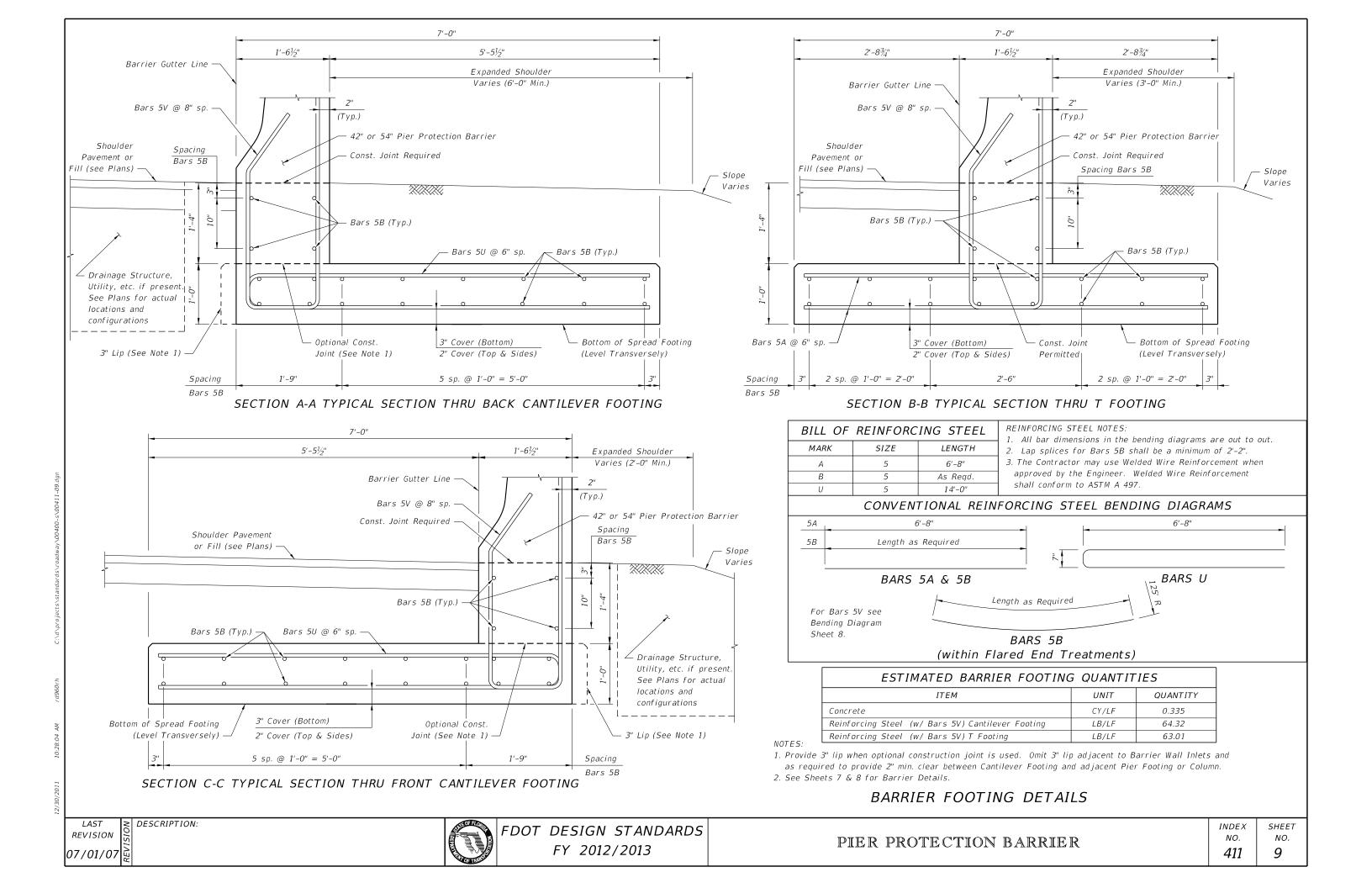
- Extend 32" Concrete Barrier Wall & supporting footing horizontal reinforcing 3' into 42" or 54" Pier Protection Barrier (Typ.) - 32" Concrete Barrier Wall, See Index 410

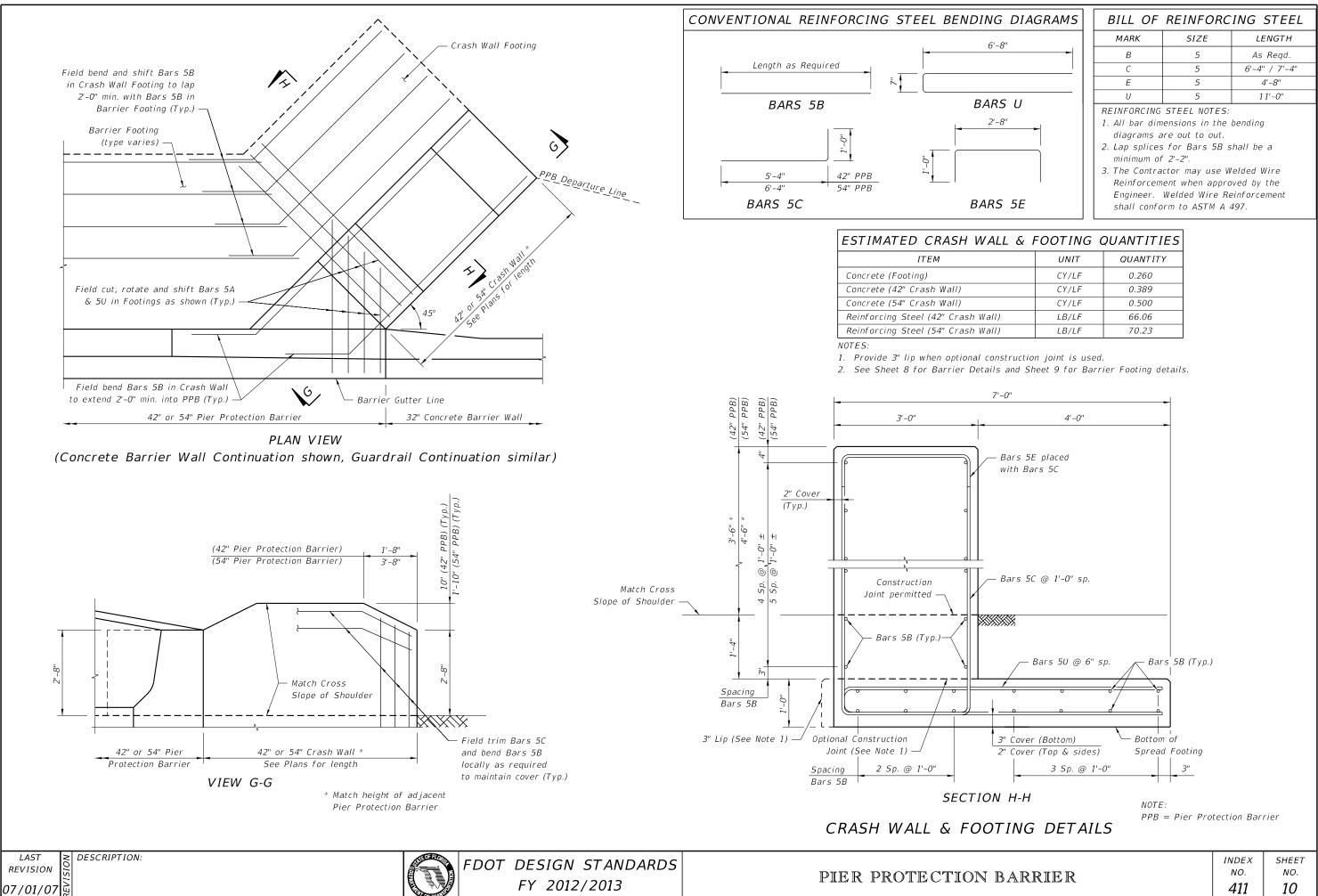
> NOTE PPB = Pier Protection Barrier.

	INDEX NO.	SHEET NO.
BARRIER	NO.	NO.
	411	6









L & I	FOOTING (	QUANTITIES
	UNIT	QUANTITY
	CY/LF	0.260
	CY/LF	0.389
	CY/LF	0.500
	LB/LF	66.06
	LB/LF	70.23

## GENERAL NOTES

- 1. The 'Portable Temporary Low Profile Barrier For Roadside Safety' is a licensed design by the University Of Florida. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing (Index No. 412) is provided by the Florida Department Of Transportation solely for use by the Department and its assignees. The purpose for this standard drawing is to indicate the approval of use of the barrier on the State Highway System; to provide sufficient pictorials for identifying the barrier unit; and, to provide general installation geometry for the barrier.
- 3. Only those barrier units cast by producers licensed by the University Of Florida will be allowed for installation on the State Highway System in Florida. Barrier wall units shall conform to Section 521 of the Standard Specification and shall be produced in Department approved plants with quality control plans for precasting concrete barrier walls. Each barrier wall unit shall be permanently marked with an identification that is traceable to the manufacturer, the producing precast concrete plant and the date of production. This permanent identification mark will serve as certification that the unit has been manufactured in accordance with University of Florida drawings and specifications, and the approved quality control program.
- 4. The low profile barrier is to be installed only with hardware and accessories furnished by the licensed barrier producer. Units shall be used for no purpose other than as interconnected segments in a run of barrier. Low profile barrier wall units shall maintain firm contact with adjoining units. Nuts on tensioning rods shall be installed snug tight.
- 5. The low profile barrier is applicable for work zone speeds of 45 mph or less.
- 6. Tubular markers shall be installed along the run of barrier at the ends and at 50' centers on tangents and 25' centers on radii. The markers shall be fixed to the top of the barrier by an adhesive or other method approved by the engineer. Approach end units shall be marked with a Type I object marker. The cost of the tubular markers and Type I object marker shall be included in the cost of the low profile barrier.
- Information regarding licensing, shop drawings, specifications, quality control and certification of compliance can be obtained from the University Of Florida: Office of Technology Licensing, P.O. Box 115500, Gainesville, Florida, 32611-5500. Telephone: 352-392-8929, Fax: 352-392-6600. Reference UF#11052.
- 8. The Portable Temporary Low Profile Barrier For Roadside Safety shall be paid for under the contract unit price for Barrier Wall (Temporary) Low Profile Concrete, LF, and will be full compensation for furnishing, installing, maintaining and removing barrier wall.

BACKSIDE	AND	END	PICTORIAL	VIEWS

PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

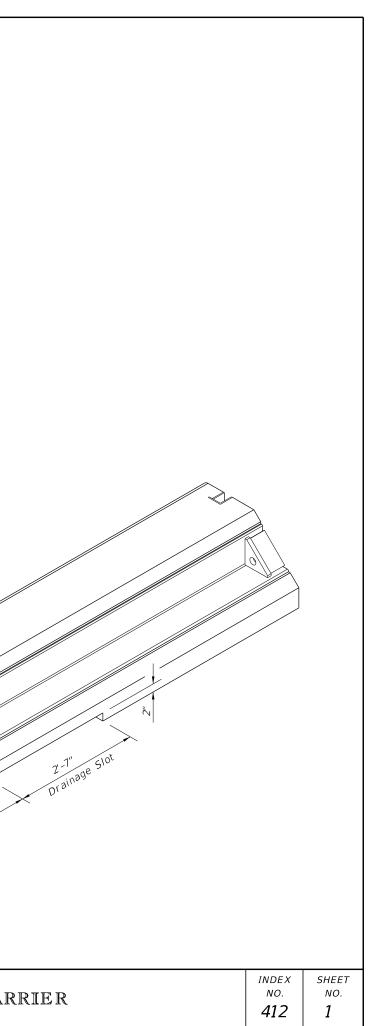
Unit Length 12,00

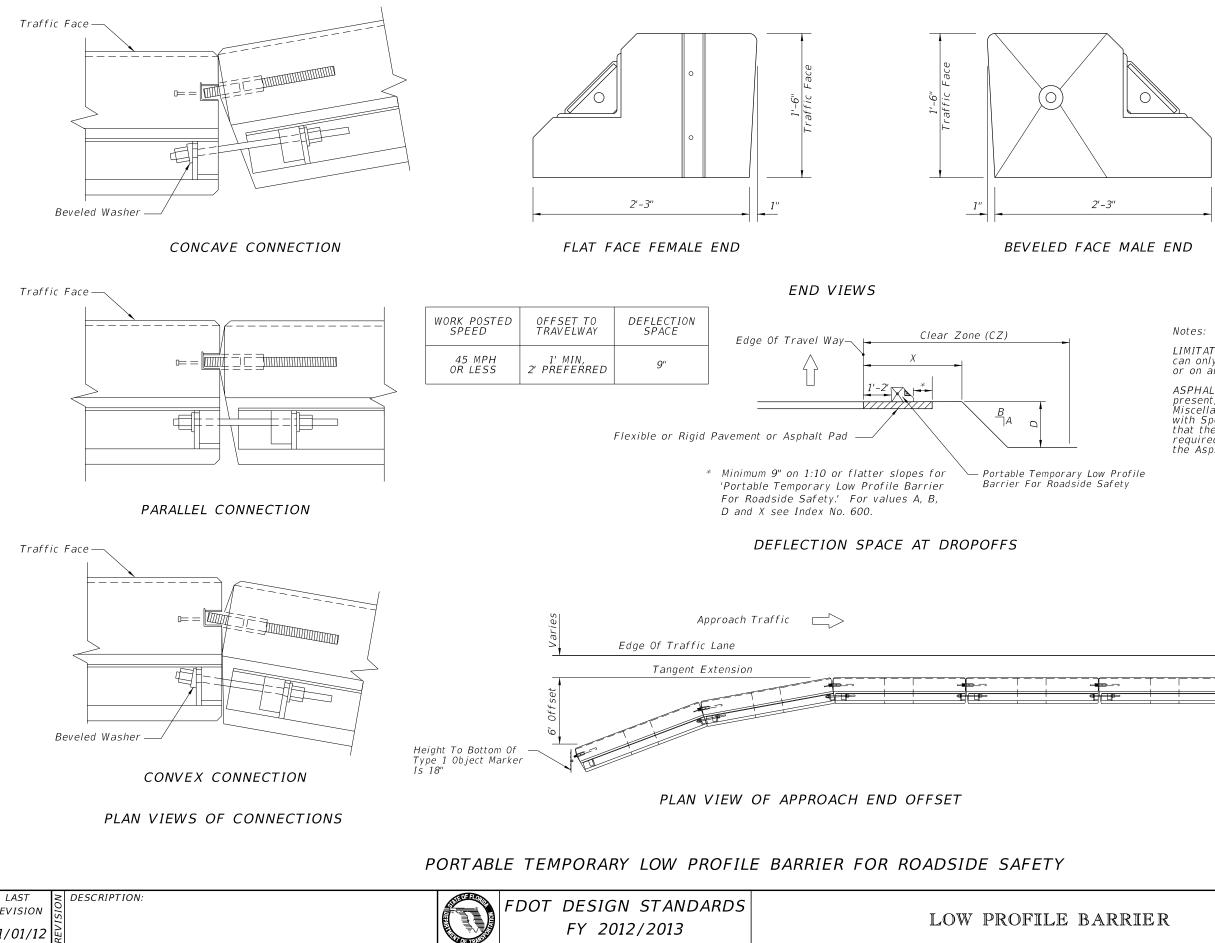
	FL

FDOT DESIGN STANDARDS FY 2012/2013

LOW PROFILE BARRIER

4-81/2



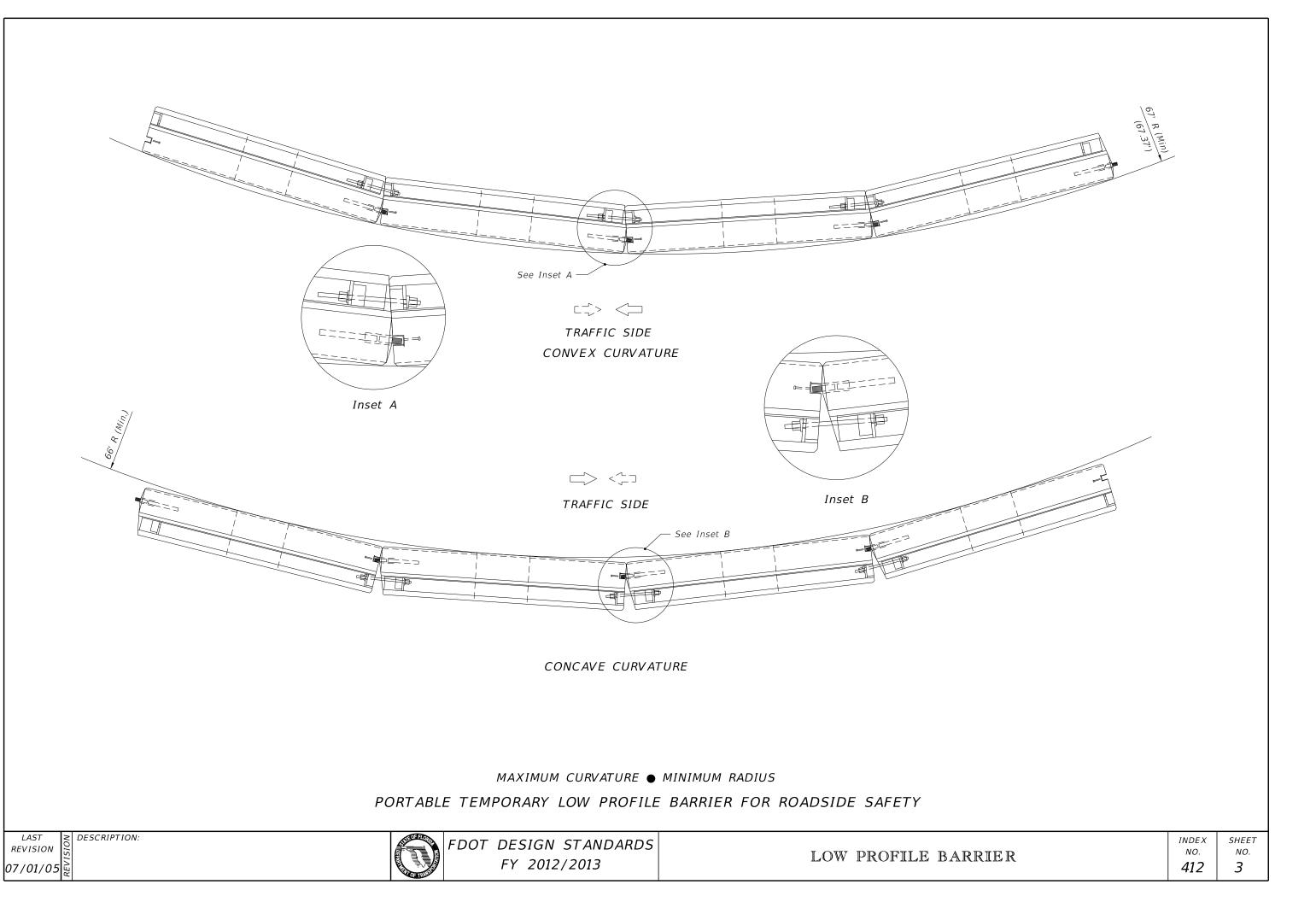


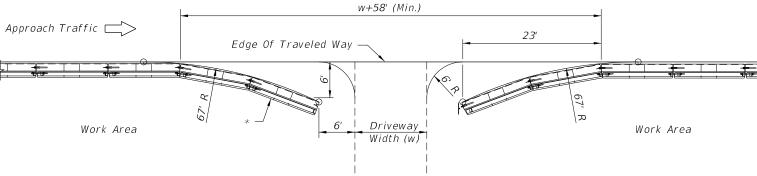
REVISION 01/01/12

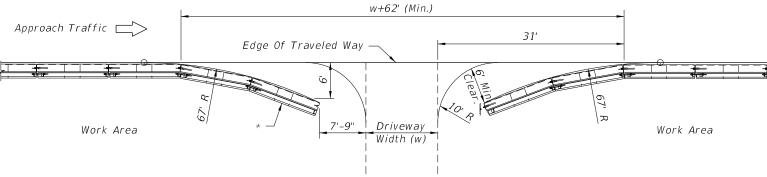
LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement or on an asphalt pad as shown.

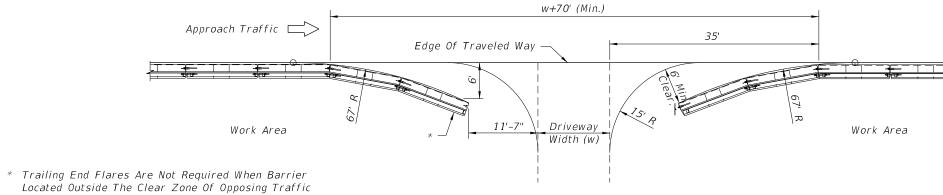
ASPHALT PAD: Where exisiting pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

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Type I Object Marker To Be Installed When Trailing End Flare Falls Within The Clear Zone Of Opposing Traffic

# LEGEND

BARRIER OPENINGS AT DRIVEWAYS

Type I Object Marker

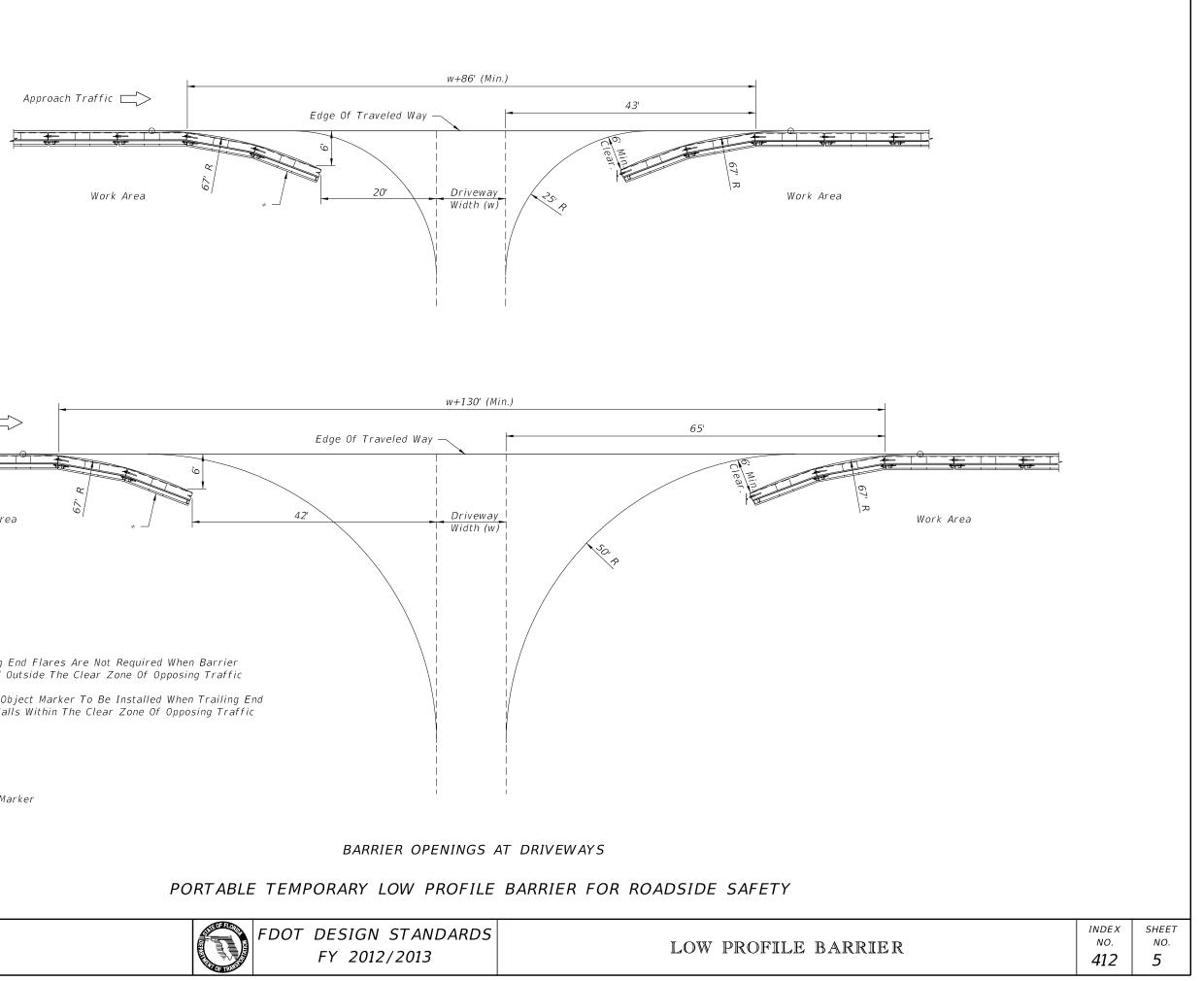
PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY

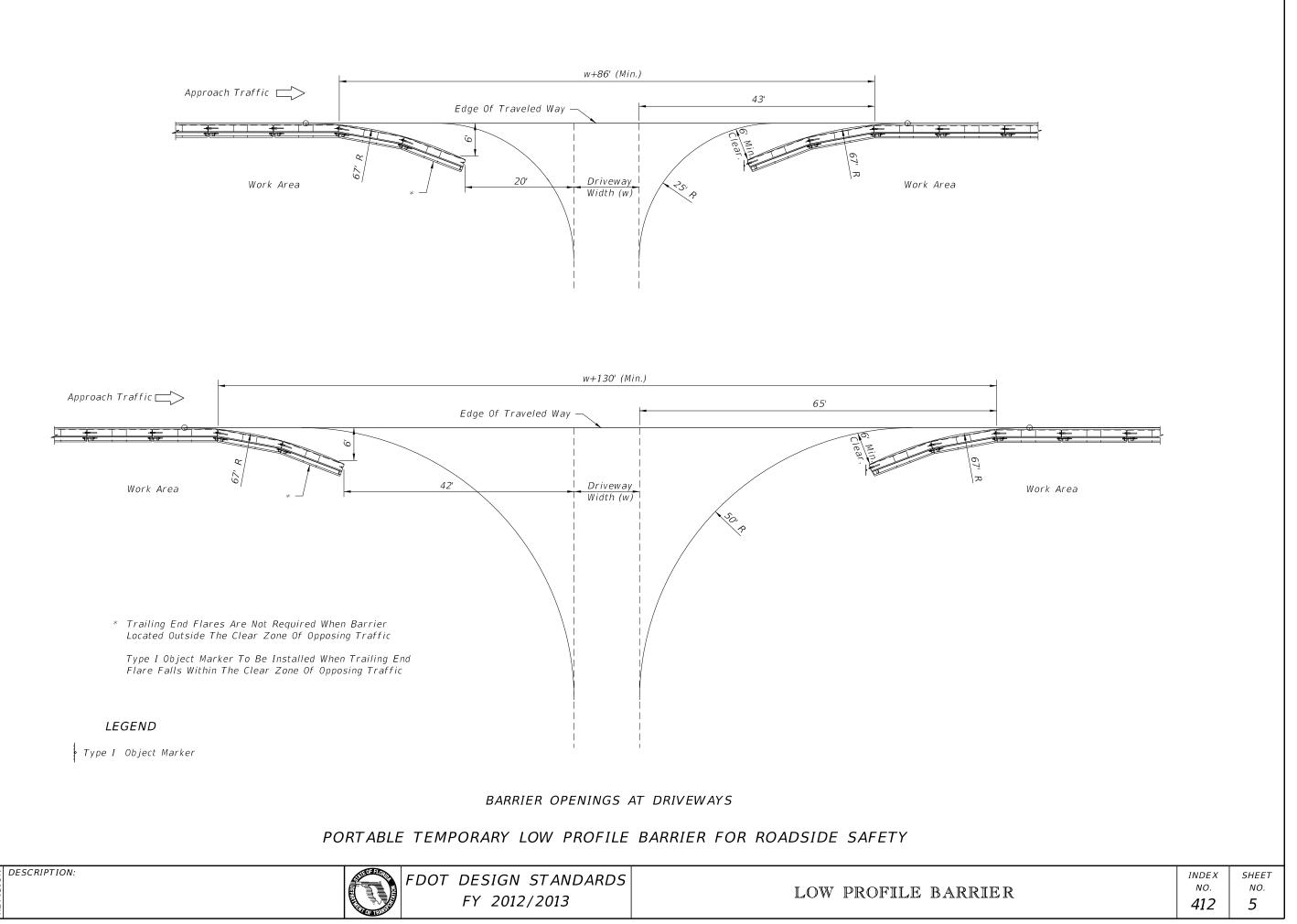
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LOW PROFILE BA

RRIER	index no. <b>412</b>	sheet NO. <b>4</b>





LAST

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01/01/12

The Type K Temporary Concrete Barrier System has been crash tested to NCHRP Report 350 TL-3 criteria or structurally evaluated to meet the requirements of NCHRP Report 350 TL-3 criteria for the installation configurations as shown utilizing the types, sizes, lengths, shapes, strengths and grades of the fabrication and installation materials as shown.

In order to maintain crashworthiness of the system, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

## FABRICATION NOTES:

- FABRICATOR PREQUALIFICATION: The Barrier Units shall be made in a prestressed concrete plant that meets the requirements of Specification Section 450 or in a precast plant meeting the requirements of Specification Section 6-8.
- CONCRETE: Concrete shall be Class IV in accordance with Specification Section 346. Specification Sections 346–10.2 through 346–10.4 are not applicable. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.
- REINFORCING STEEL: All reinforcing steel shall be ASTM A 615, Grade 60 except for Bars 6D1, 6D2 and 6D3. Bars 6D1, 6D2 and 6D3 shall be ASTM A 706 except that a 2¾" diameter pin must be used for the 180 degree bend test. After fabrication, all or part of Bars 6D shall be hot dip galvanized in accordance with Specification Section 962 or coated with a cold galvanizing compound in accordance with Specification Section 975. The minimum limit of galvanizing or coating is shown in the Bending Diagrams. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated. Install Bars 6D within ½" of the plan dimensions. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.

At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with ASTM A 497 and the details shown on Sheet 2 may be utilized in lieu of Bars 4A and 5B.

All dimensions in the Bending Diagrams are out to out. All reinforcing steel shall have a 2" minimum cover except as noted.

- LIFTING SLEEVE ASSEMBLY: Inclusion of the Lifting Sleeve Assemblies is optional. Steel for Pipe Sleeve shall be in accordance with ASTM A 53. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.
- SURFACE FINISH: Construct Barrier Units in accordance with Specification Sections 400 and 521. Finish the top and sides of the Barrier Units with a General Surface Finish. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish. Use stationary metal forms or stationary timber forms with a form liner.

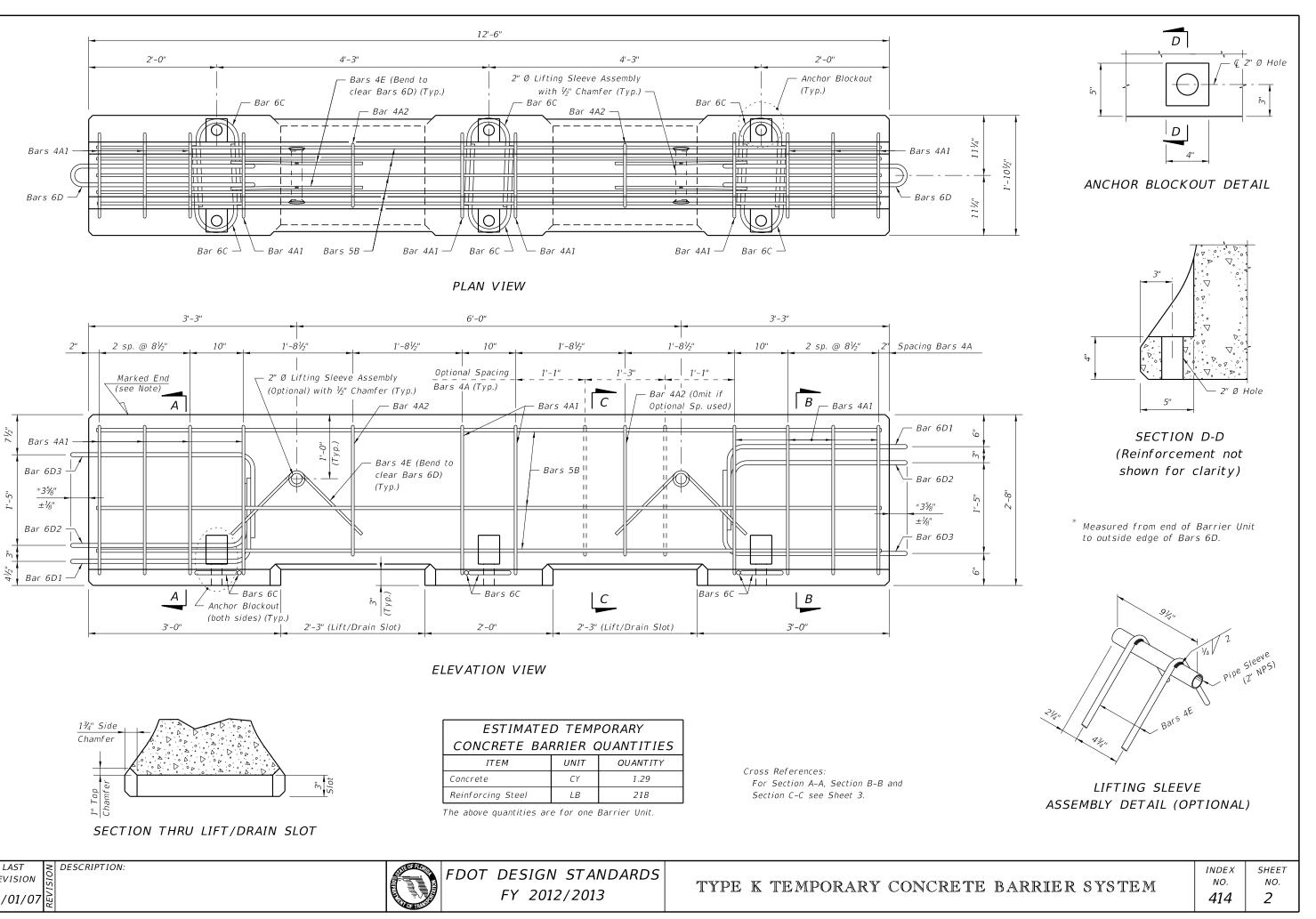
MARKING: Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall. Ink stamps are not allowed. Permanently mark with the following information:

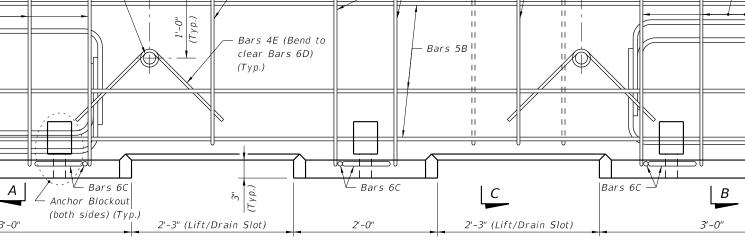
- Туре К1
- Fabricator's name or symbol
- Date of manufacture (day, month and year)
- HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

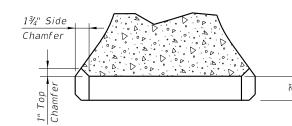
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2	BARRIER SYSTEM	index no. <b>414</b>	sheet NO. <b>1</b>

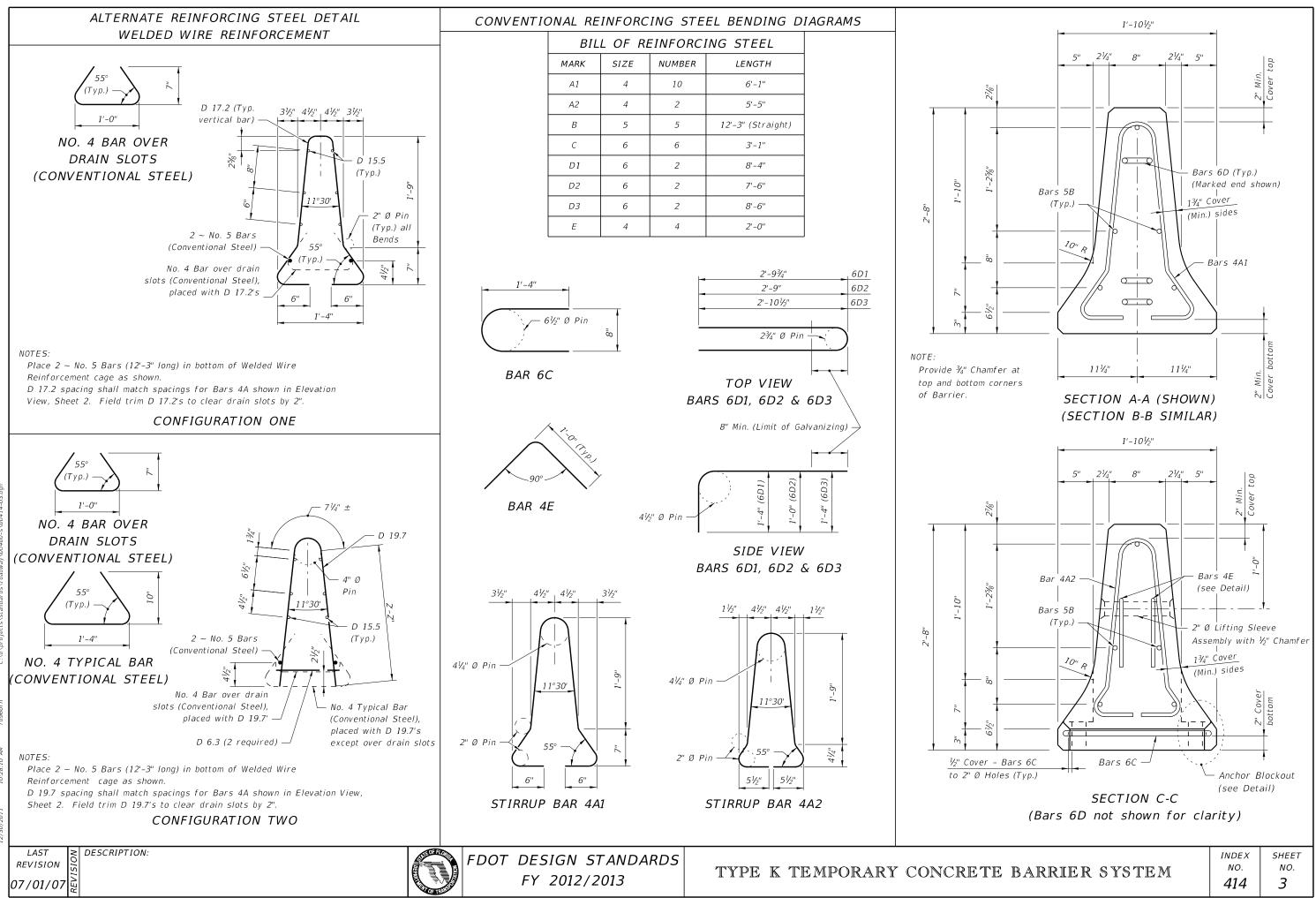


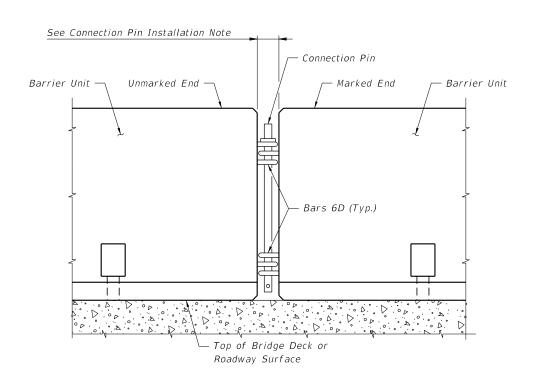




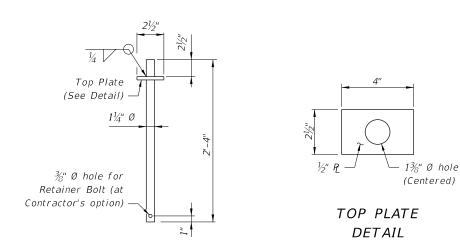
ESTIMATED TEMPORARY					
CONCRETE BARRIER QUANTITIES					
ITEM	UNIT	QUANTITY			
Concrete	СҮ	1.29			
Reinforcing Steel	LB	218			







DETAIL OF CONNECTION BETWEEN BARRIER UNITS



CONNECTION PIN DETAIL

## NOTES FOR ALL INSTALLATIONS:

LIMITATION OF USE: This Temporary Concrete Barrier System is intended for work zone traffic control and other temporary applications. It shall not be used for permanent traffic railing construction unless specifically permitted by the Plans. Except as shown for the Back Filled Roadway Installations, the Barrier Units must be installed on a flexible pavement (asphalt) or rigid pavement (concrete) surface as shown with a cross slope of 1:10 or flatter. Except as shown for transition installations, Type K Barrier Units are not intended to be bolted down or staked down in locations where they can be impacted from the back side.

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 27 tons

SURFACE PREPARATION: Except as shown for the Back Filled Roadway Installations, remove all debris, loose dirt and sand from the pavement, bridge deck or Asphalt Pad surface within the barrier footprint just prior to placement of the Barrier Units.

CONNECTION PIN ASSEMBLY: Steel for Connection Pin and Top Plate assemblies shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Nondestructive testing of welds shall not be required. At the Contractor's option, a 💥 diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.

CONNECTION PIN INSTALLATION: Initially set Barrier Units by using a 35% wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 5). Barrier Units shall not be used unconnected.

DELINEATION: Mount Type C Steady-Burn Lights on top of Barrier Units that are used as traffic barriers along travel ways in work zones. Space the lights at 50' centers in transitions, 100' centers on curves and 200' centers on tangent alignments. Refer to "Warning Lights" on Index No. 600 for additional information.

REUSE OF UNITS: Barrier Units may be reused provided they have the structural integrity and surface qualities of new units. Do not use Barrier Units without Marking Plates.

REUSE OF CONNECTION PINS: Connection pins may be reused if they have the structural integrity of new pins.

INSTALLATIONS ON CURVED ALIGNMENTS: The details presented in these Standards are shown for installations on tangent alignments. Details for horizontally curved alignments are similar.

TRANSITIONS: Transitions are required between freestanding, bolted down, staked down and back filled Type K Barrier installations, see Sheet 8 for transition requirements and details. Transitions are also required between installations of Type K Barrier and other types of temporary barrier, see Index No. 415 for transition requirements and details. Splices and transitions are required between installations of Type K Barrier and permanent Bridge or Roadway Traffic Railings, see Sheets 9 through 13 for transition requirements and details. Transitions are required between installations of Type K Barrier and Proprietary (QPL) Barrier Systems, See Sheets 14 and 15 for transition requirements and details.

PAYMENT: Barrier Units for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary) (F&I) (Type K), LF. Any relocation of the Barrier Units required for the project shall be paid for under the contract unit price for Barrier Wall (Temporary) (Relocate) (Type K), LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights (Temp. Barrier Wall Mount) (Type C, Steady Burn), ED. The Contractor shall furnish Barrier Units except when the Plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return. Unless otherwise noted on the Plans, the BarrierUnits shall become the property of the Contractor and shall be removed from the site prior to acceptance of the completed project.

NOTES FOR THRIE BEAM GUARDRAIL SPLICE INSTALLATIONS:

THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices in accordance with AASHTO M 180, Type II (Zinc coated) and as follows: Two panels per splice (One panel per side) of Class B (10 Gauge), or

Four panels per splice (Two nested panels per side) of Class A (12 Gauge).

Guardrail panel length shall be 12'-6". Provide and install all other associated metallic guardrail components (Terminal Connectors, Shoulder Bolts, Hex Bolts and Nuts, Filler Plates, etc.) in accordance with Index No. 400.

Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted. Do not drill or cut through utilities or conduits within permanent concrete traffic railings.

GUARDRAIL OFFSET BLOCKS: Provide and install timber Offset Blocks meeting the material requirements of Index No. 400. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.

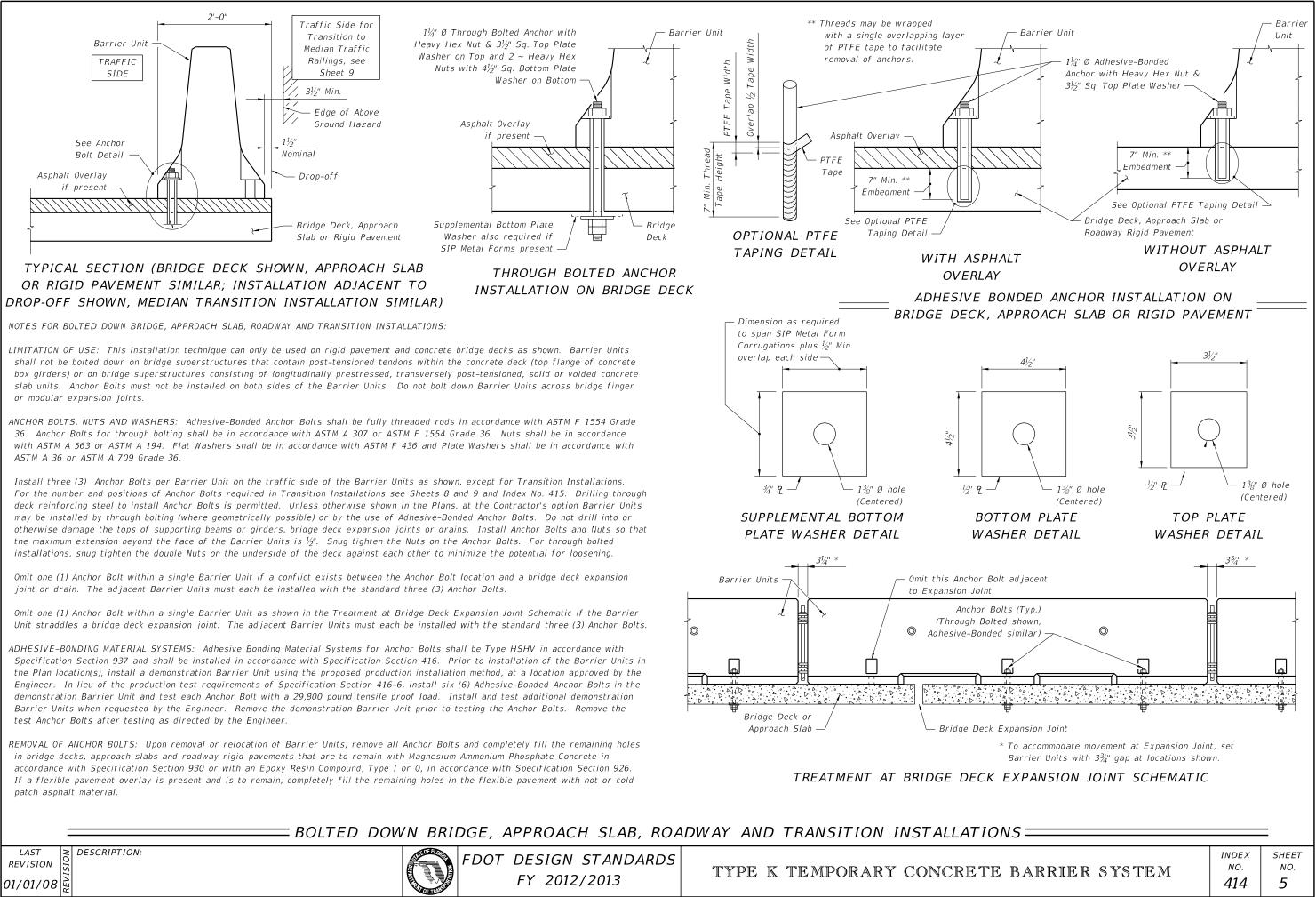
CONCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification Section 346, any Class, or a commercially available prebagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification Section 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.

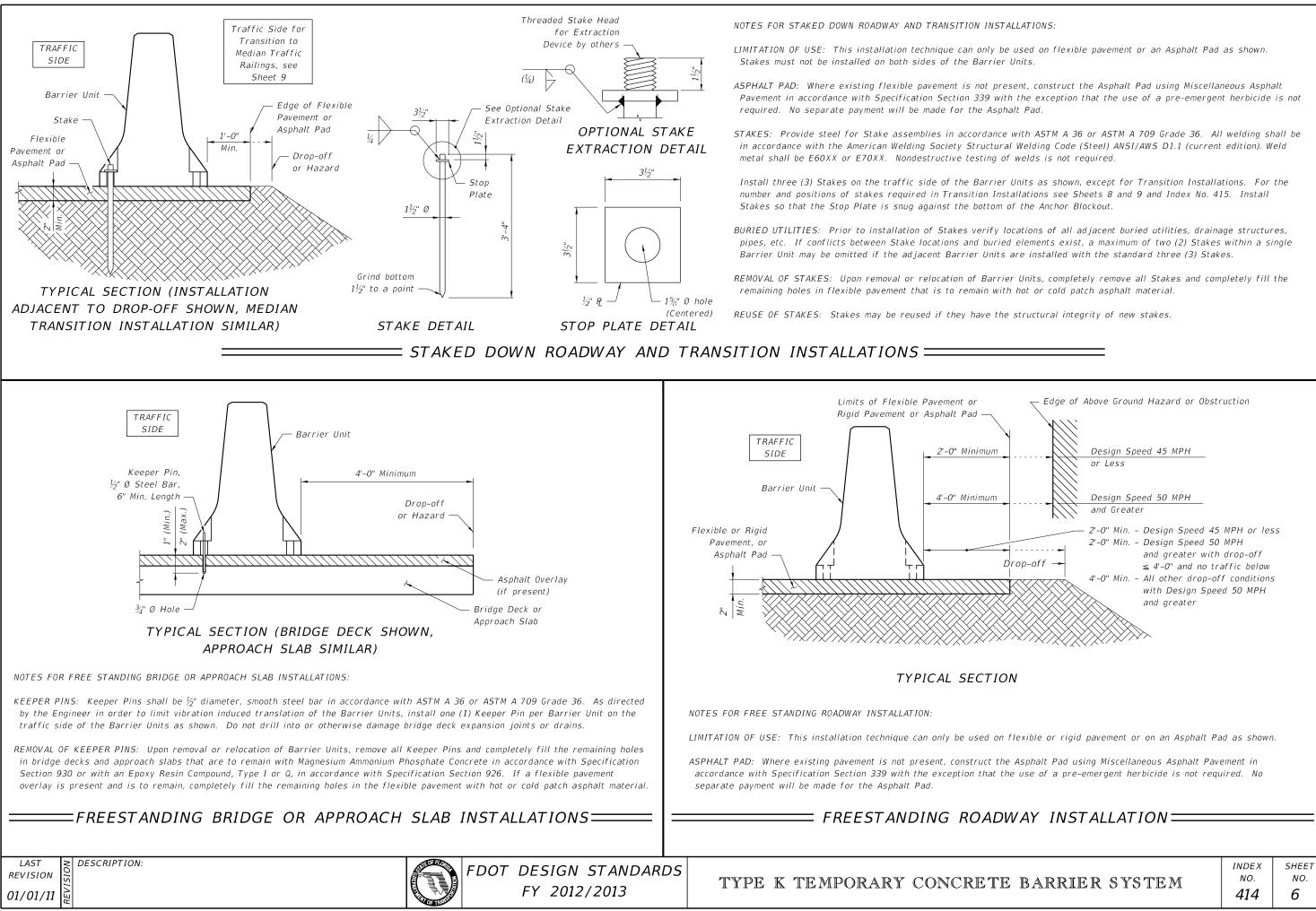
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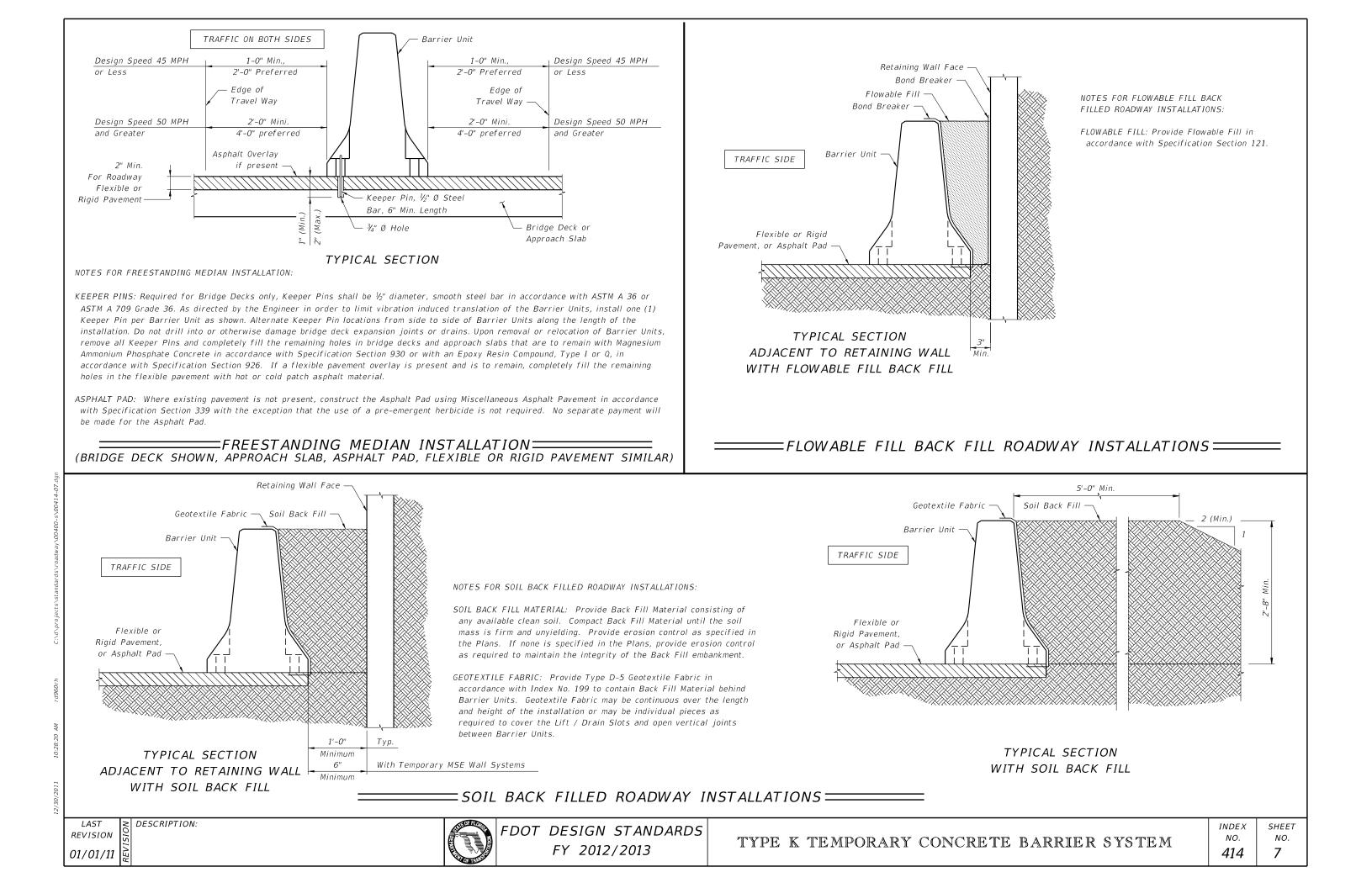
TYPE K TEMPORARY CONCRETE

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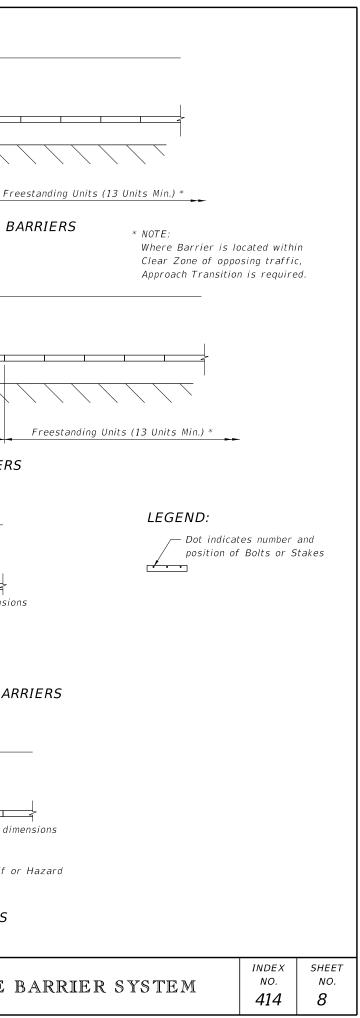


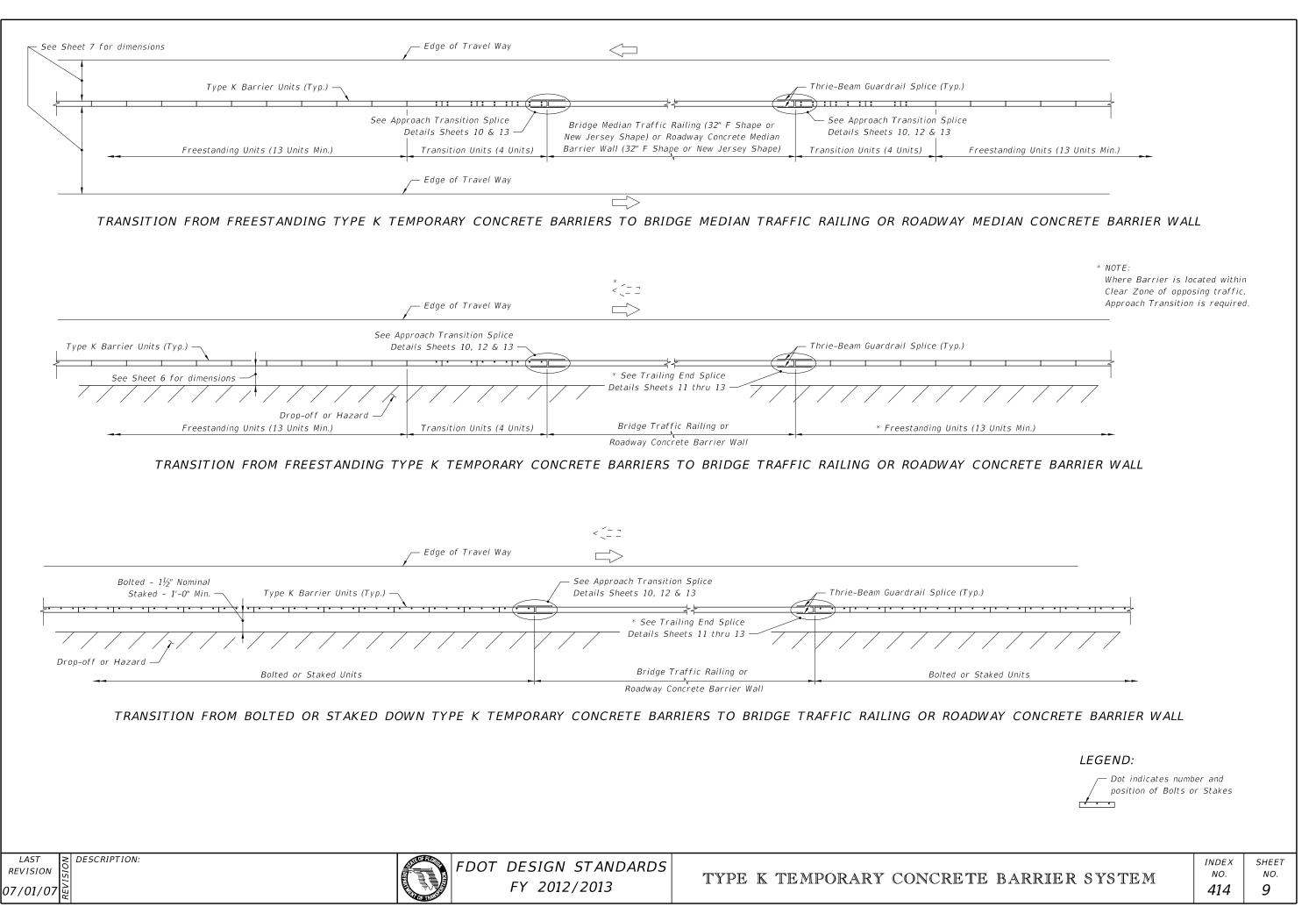


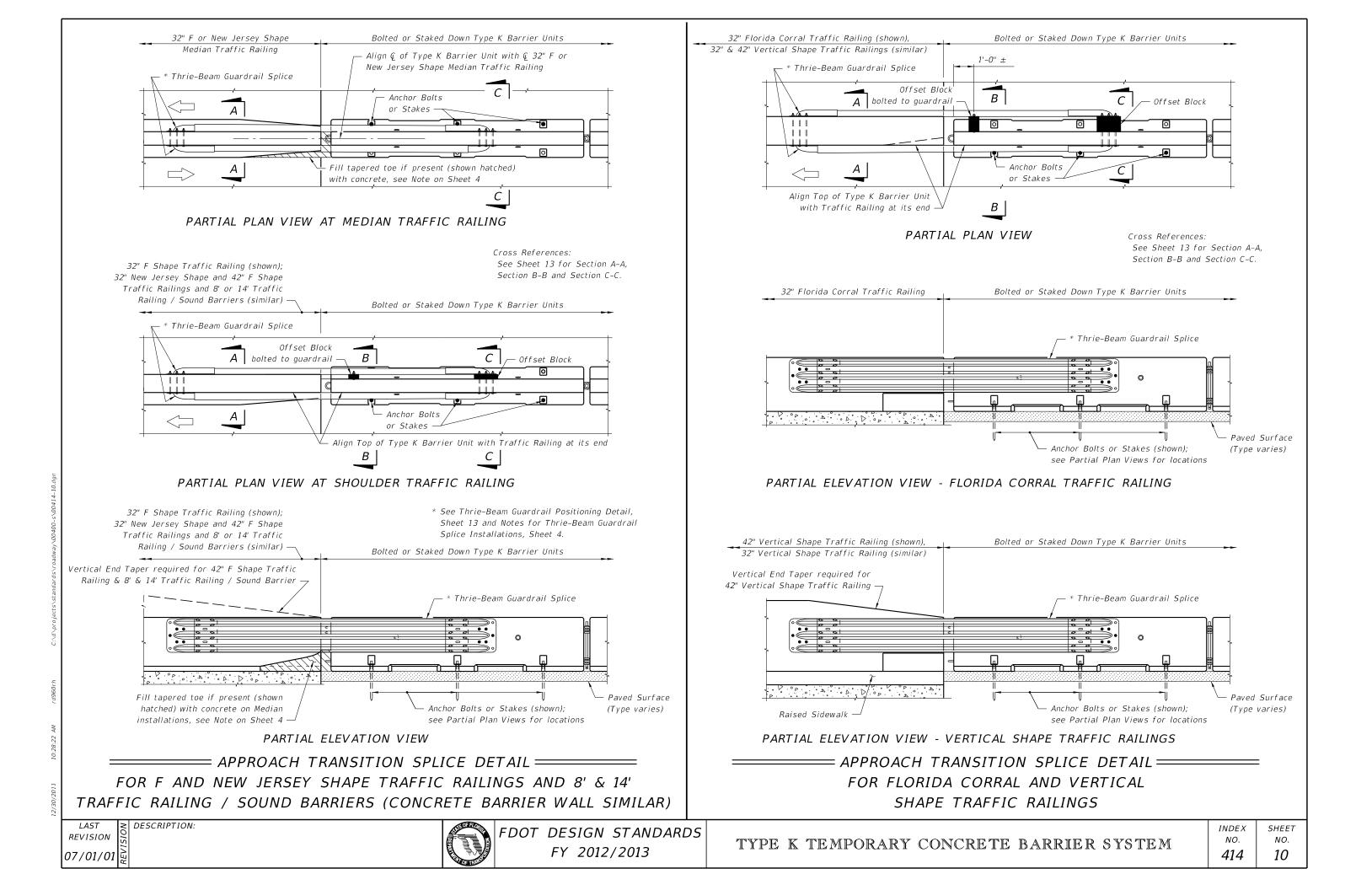
	INDEX	SHEET
BARRIER SYSTEM	NO.	NO.
	414	6

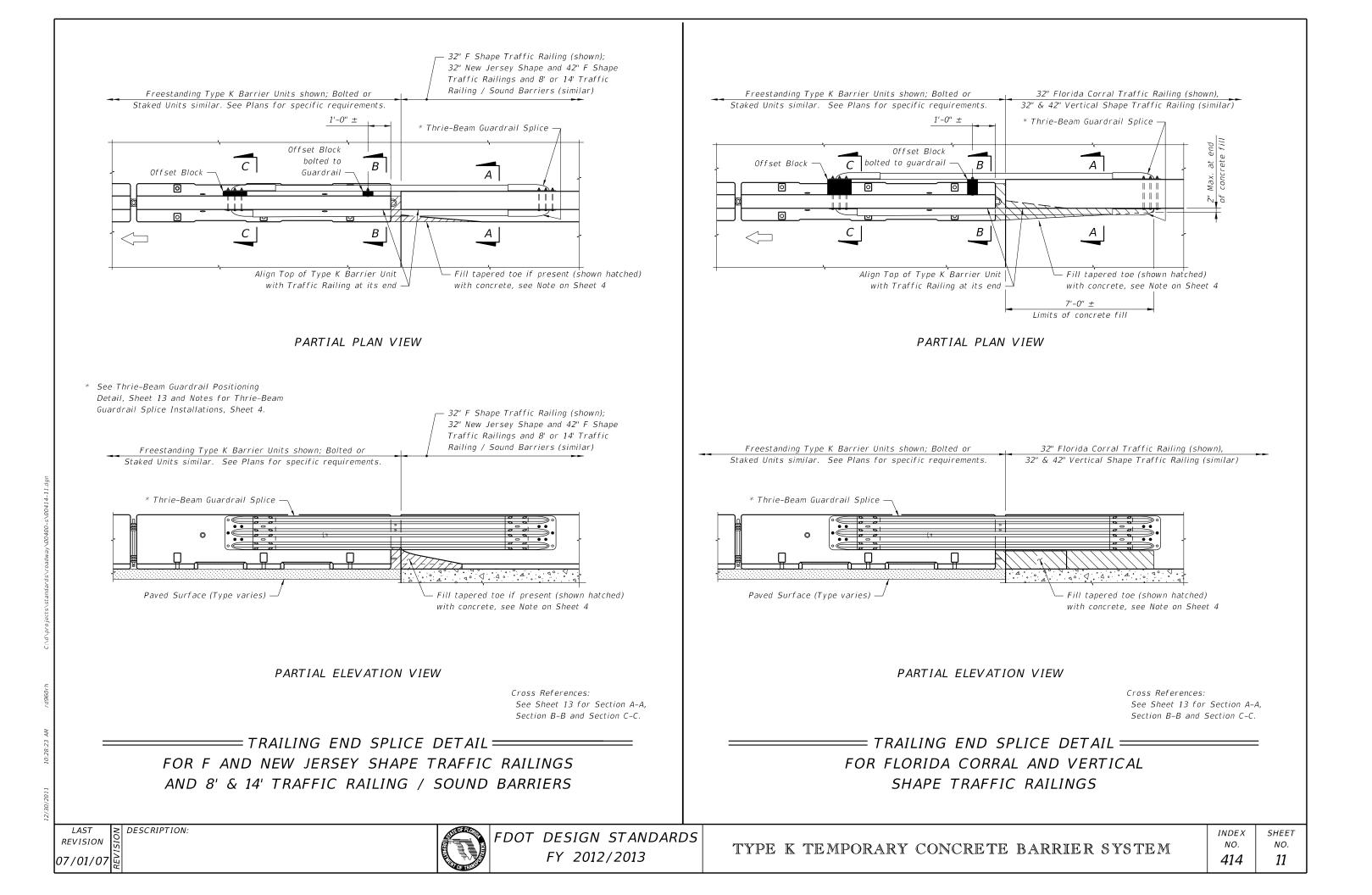


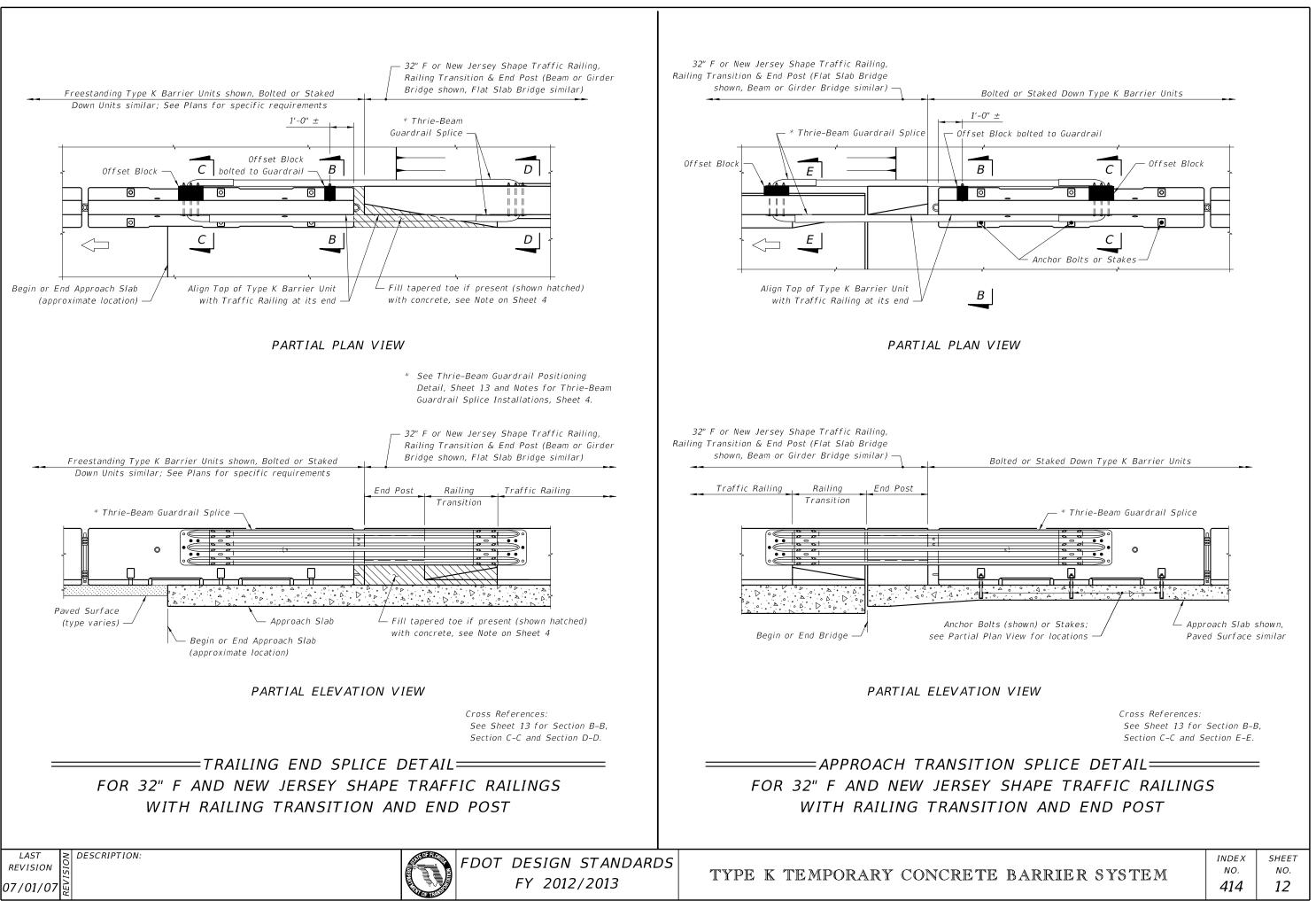
		*			
			E	Edge of Travel Way —	
				<b>\</b>	
	_	First full Barrier Unit L	pefore Dron-off or	∕— Bolted – 1½″ Nominal	
Туре К Ва	/	Hazard shielded by Bol		Staked – 1'–0" Min.	
		· · ·   · · ·   · · ·		••••	<u> </u>
See Sheet 6 for dimensions —		$\square / $			+
		Drop	o-off or Hazard		
Freestanding Units (13 Units Min.)	Transition Units (4 Units)	Bolteo	or Staked Units	Transition Units (4 Units) *	F
					- <b></b>
APPROACH TRANSITION	FROM FREESTANDING TO	BOLTED OR STA	AKED DOWN TYP	PE K TEMPORARY CONCRE	TE
		* ~			
		* ~	F	Edge of Travel Way —	
			L		
— Туре К Barri	ier Units (Typ.)	arrier Unit before Back	Filled Units		
See Sheet 6 for dimensions —					
//////////////////////////////////		Back Fill —			$\langle \rangle$
Drop-off or Hazard —/					
Freestanding Units (13 Units Min.)	Transition Units (4 Units)	Datk	Filled Units	Transition Units (4 Units	5)*
First full B	arrier Unit after Drop-off or Hazard shielded by Bolted or Staked Units	—	oe K Barrier Units (Typ.,	)	
[••••]••	••••••••••••••••••	· · · · · · · · · · · · · · · · · · ·		Į Į I	
Bolted – $1\frac{1}{2}$ " Nominal	///////////////////////////////////////			See Sheet 6 for a	limens _
Staked _ 1'-0" Min -	prop-off or Hazard —			///////////////////////////////////////	
	Delled an Chalad Unite				
	Bolted or Staked Units		Freestanding Units	, 	
TRAILING END TRANSITION FI	ROM BOLTED OR STAKED	DOWN TO FREE.	STANDING TYPE	K TEMPORARY CONCRET	E BA
				Edge of Travel Way —	
				<b>X</b>	
		Tyl	pe K Barrier Units (Typ.,	)	
				See Sheet 6	
					<u></u>
	Back Fill —/				op-off
	Back Filled Units		Freestandi		
TRAILING END TRANST	TION FROM BACK FILLED	TO FREESTANDI	NG TYPE K TEM	PORARY CONCRETE RADD	IFDC
TRAILING END TRANSI	I TON I NOM DACK TILLED	TO TREESTANDI		I UNANI CUNCILIL DARKI	LNJ
		T			
DESCRIPTION:	FDOT DESIGN S	STANDARDS	ידריות או הוועערון		فللمللم
	FY 2012/	2013	IIPE K TE	EMPORARY CONCRE	ΙĽĽ

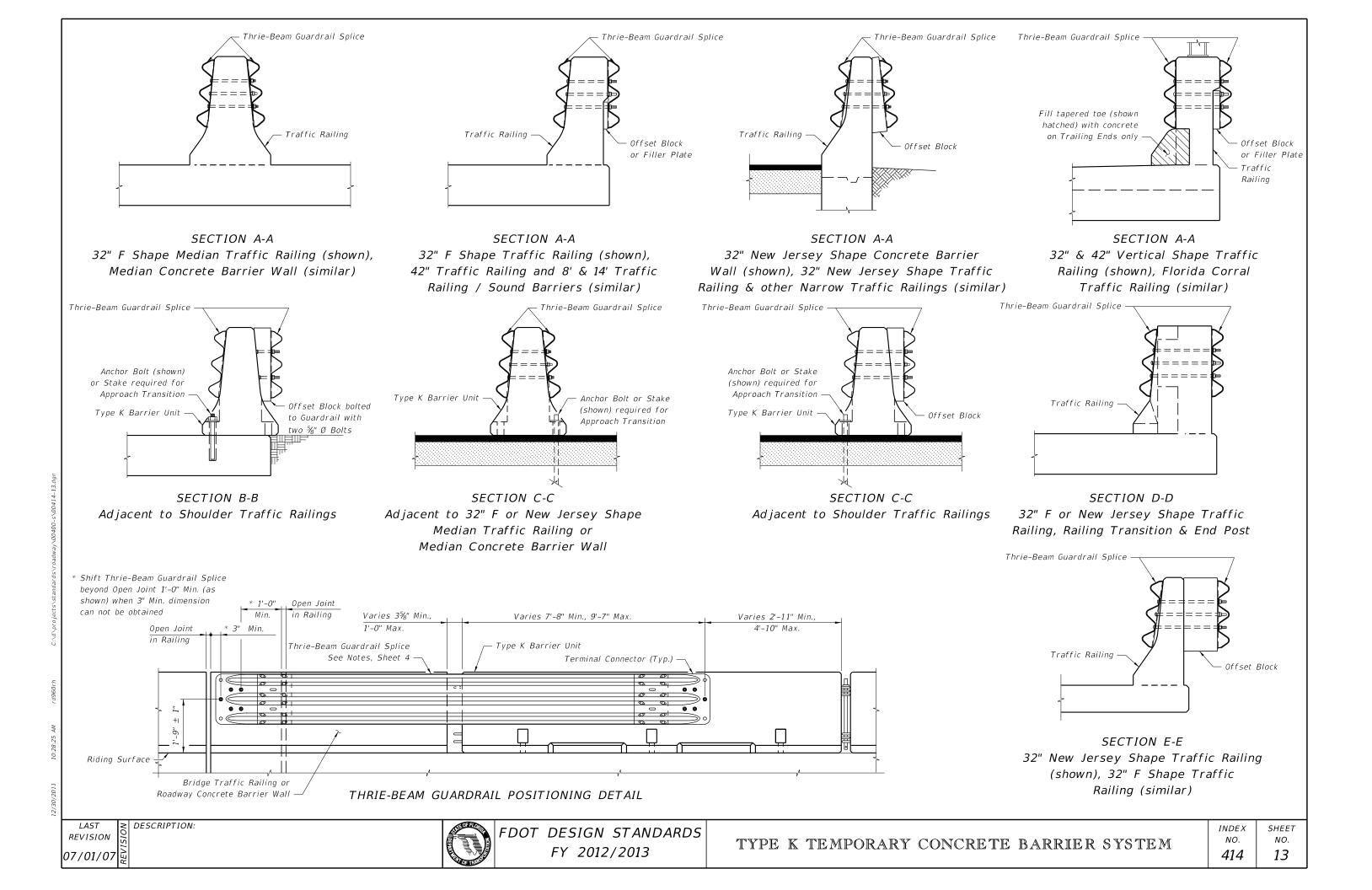


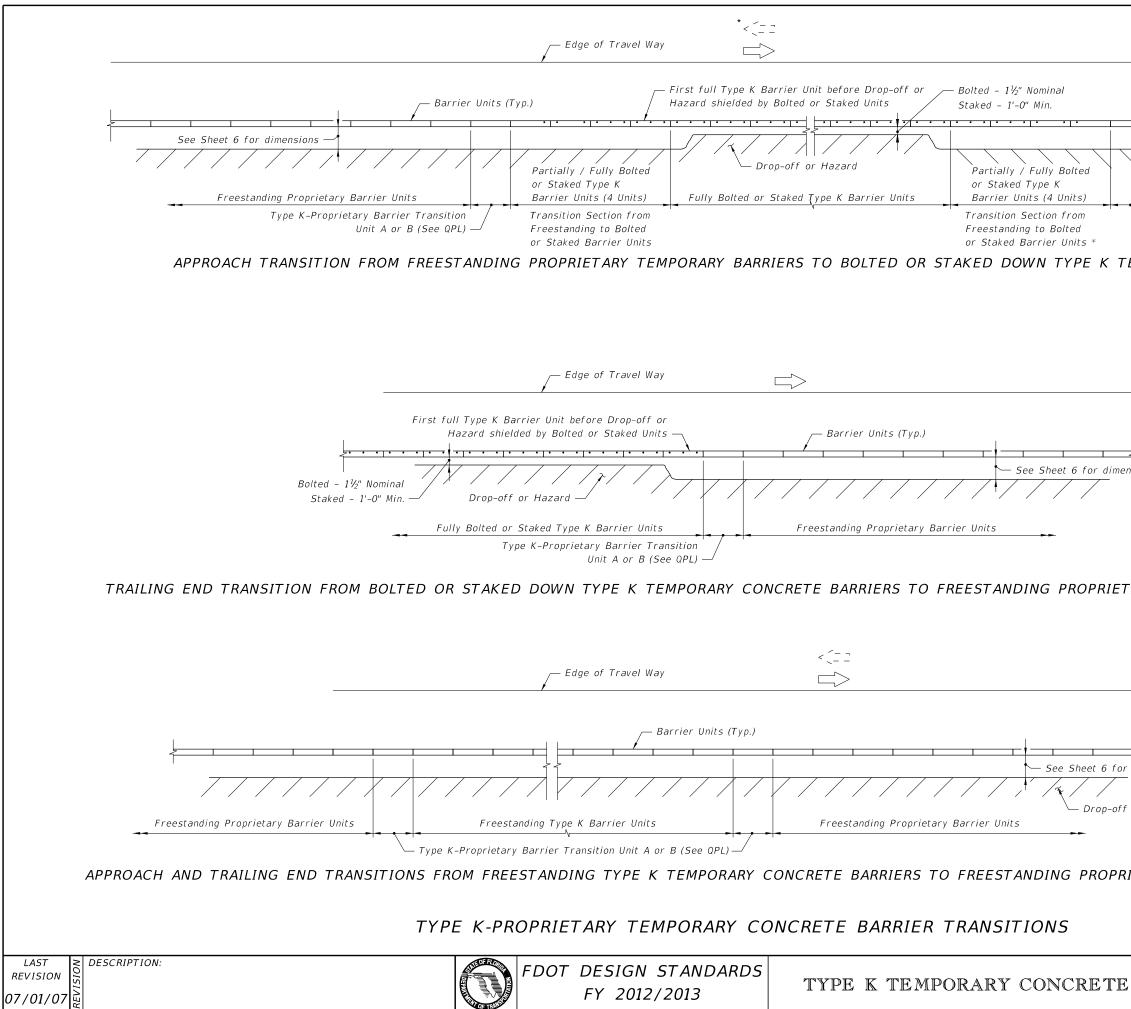




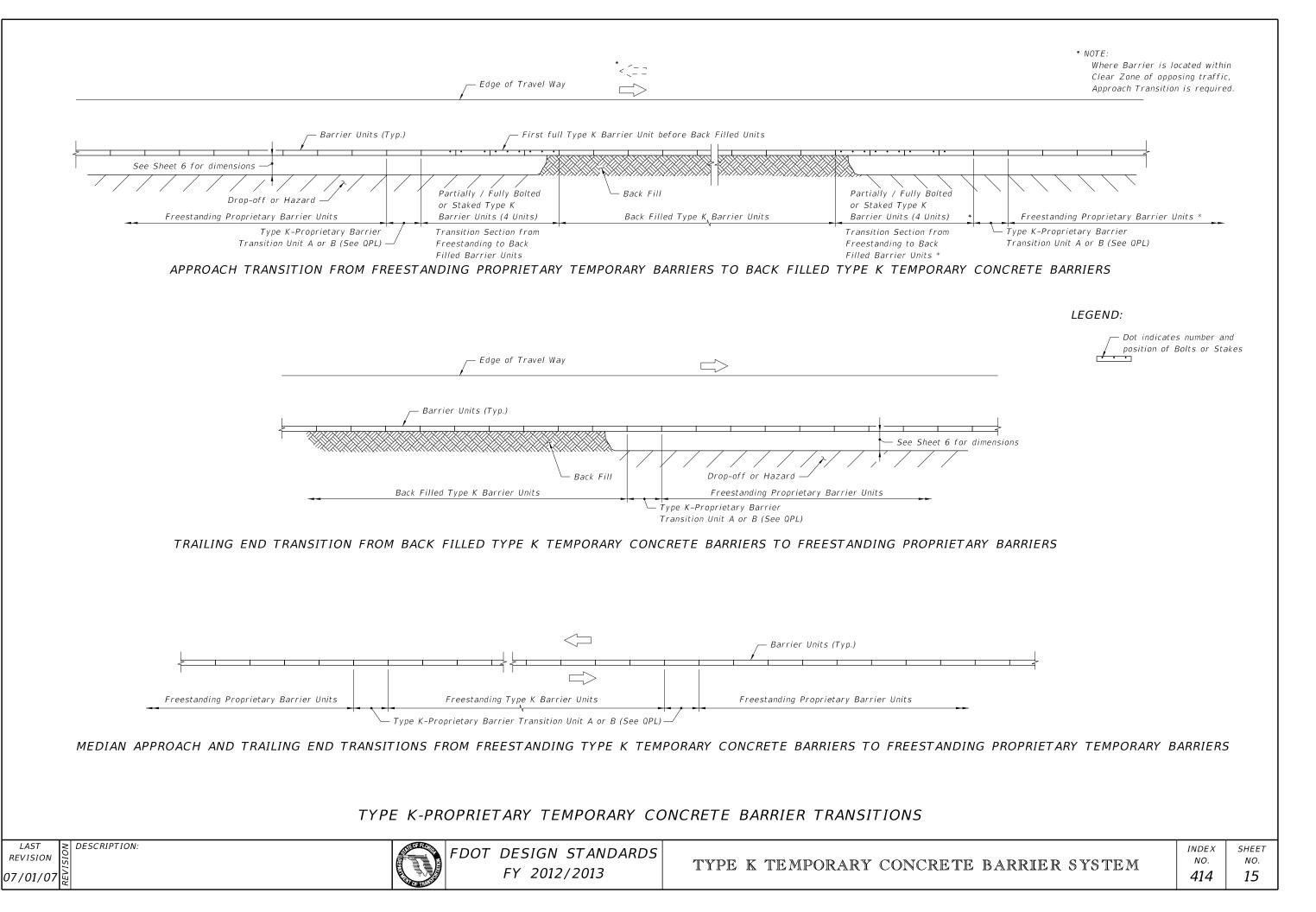






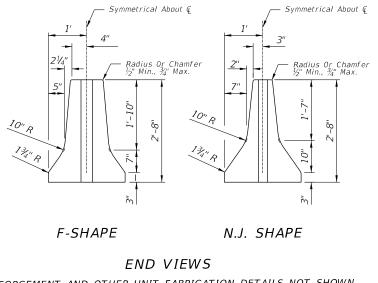


* NOTE: Where Barrier is located within Clear Zone of opposing traffic, Approach Transition is required.					
Freestanding Proprietary Barrier Units * Type K-Proprietary Barrier Transition Unit A or B (See QPL)					
EMPORARY CONCRETE BARRIERS					
LEGEND:					
	es number Bolts or S				
 ensions					
TARY TEMPORARY BARRIERS					
r dimensions					
f or Hazard					
RIETARY TEMPORARY BARRIERS					
E BARRIER SYSTEM	index NO. <b>414</b>	sheet no. <b>14</b>			

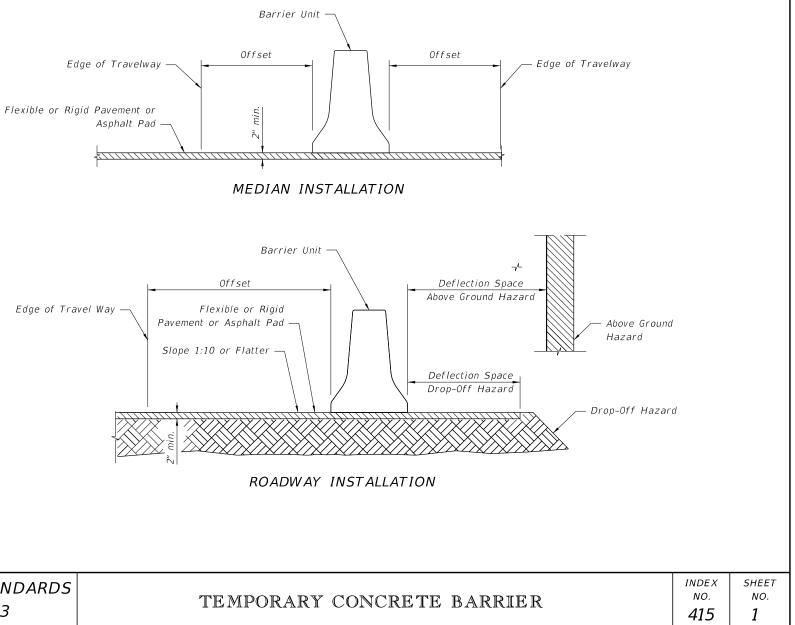


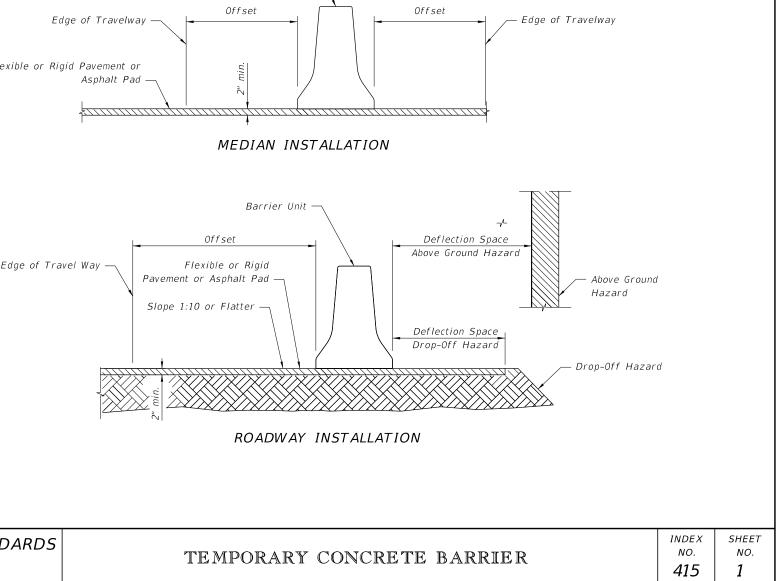
## GENERAL NOTES

- 1. Temporary concrete barrier systems on roadways may be any of the following:
- a. The FDOT Type K Temporary Concrete Barrier system (Design Standard Index 414). F-Shape Units. For temporary concrete barrier systems on bridges see Design Standard Index No. 414.
- b. Proprietary temporary concrete barrier systems meeting NCHRP Report 350 Test Level 3 criteria which are included on the Qualified Products List.
- 2. Barrier units of dissimilar types may be interconnected within a single line barriers using transition units.
- 3. Alignment, length of need, anchorage and end treatment shall be in accordance with this Index.
- 4. Temporary concrete barrier units shown herein shall not be used for permanent barrier wall construction regardless of unit length.
- 5. If the plans specify Barrier (Temporary) (Type K), substitution with other barrier types is not permitted.
- 6. If the plans specify temporary concrete barrier system, substitution with water filled barriers is not permitted.
- 7. Where existing pavement is not present, construct an Asphalt Pad using Miscellaneous Asphalt Pavement. Cost of the Asphalt Pad to be included in the cost of the Barrier system.
- 8. Type C Steady-Burn Lights are to be mounted on top of temporary concrete barriers that are used as barriers along traveled ways in work zones. The lights are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways. For additional information refer to Index 600.
- 9. Barrier units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier (Temporary), LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights, Temporary, Barrier Mount (Steady-Burn), ED.
- 10. Deflection space shall be clear of any construction debris, stockpiled materials, equipment, and objects.



REINFORCEMENT AND OTHER UNIT FABRICATION DETAILS NOT SHOWN. PERMITTED BARRIER UNIT END VIEWS





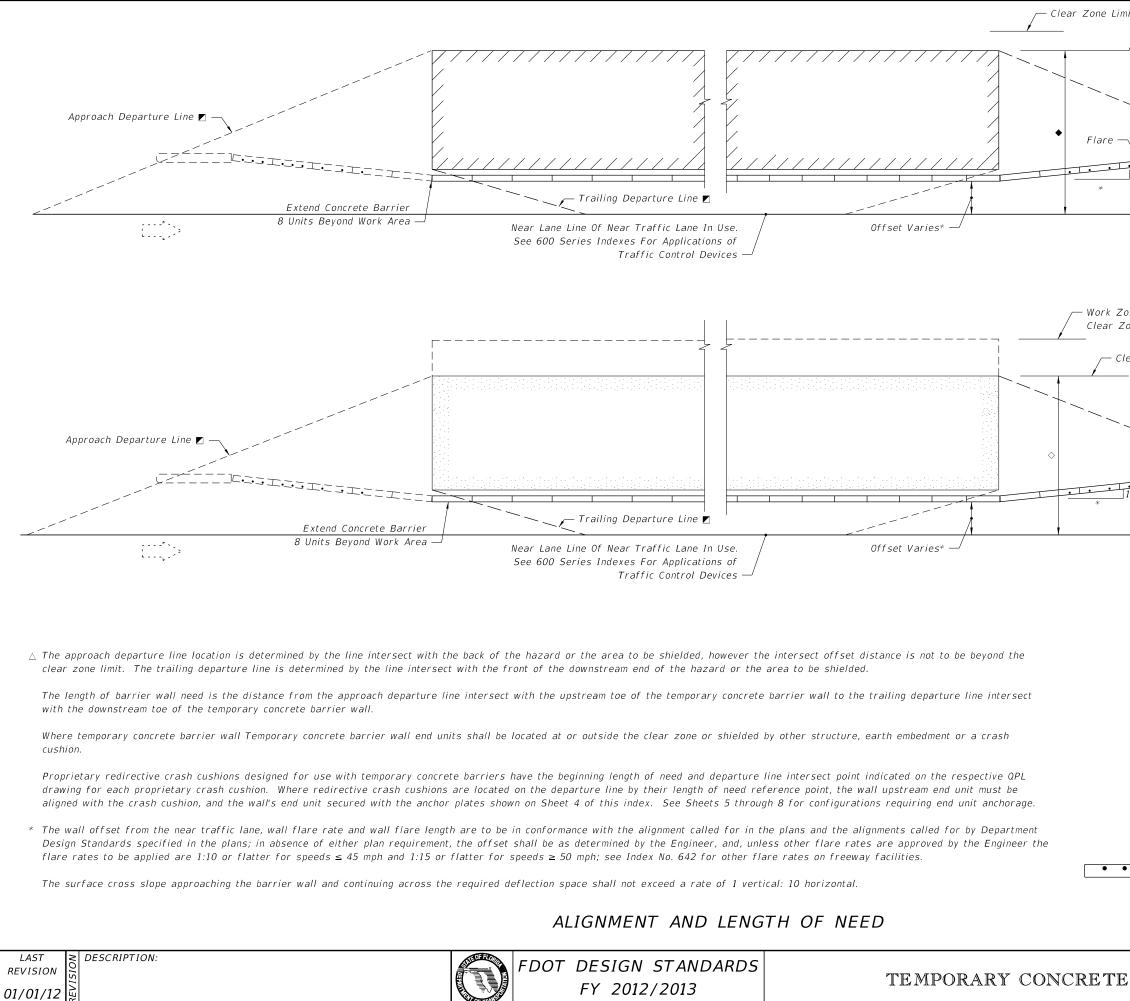
	OFI	SET AND DEFLECTION SPACE REQUIRE	MENTS	
Installation	Shielding	Work Zone Speed	Offset to Travelway	Deflection Space
	Above Ground	45 mph or Less	1' min, 2' preferred	2' min.
	Hazards	50 mph and Greater	2' min, 4' preferred	4' min.
Right Shoulder		45 mph or Less	1' min, 2' preferred	2' min.
Shoulder	Drop-Off	50 mph and Greater		
	Hazards	a. Drop-offs 4' or Less and NO traffic below	2' min, 4' preferred	2' min.
		b. All drop-off conditions other than 'a'	2' min, 4' preferred	4' min.
Separating	Ad jacent Opposing	45 mph or Less	1' min, 2' preferred	1' min., 2' prefered
Traffic	Traffic	50 mph and Greater	2' min, 4' preferred	2' min., 4' preferred

DESCRIPTION: LAST REVISION 01/01/12

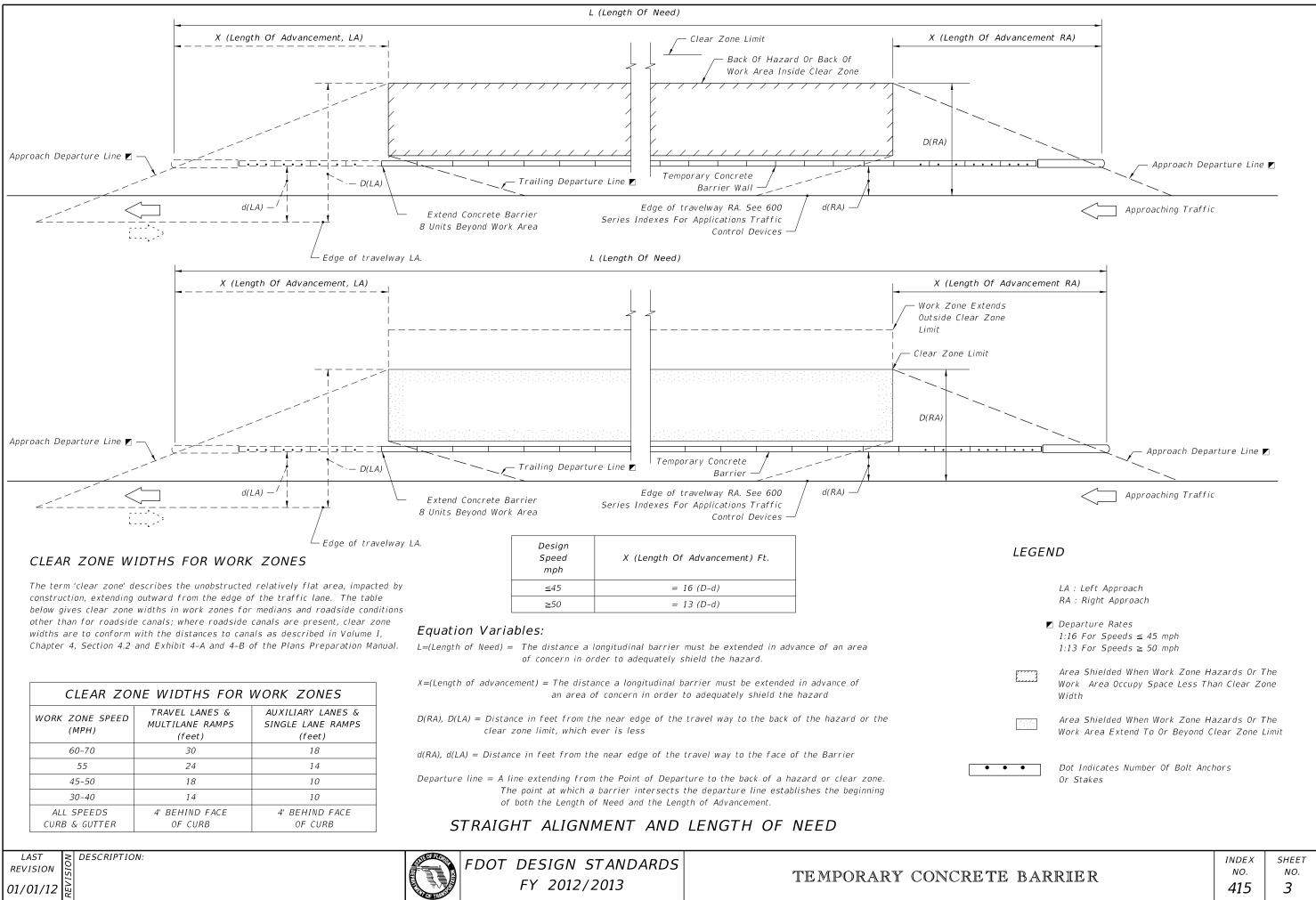


FDOT DESIGN STANDARDS FY 2012/2013

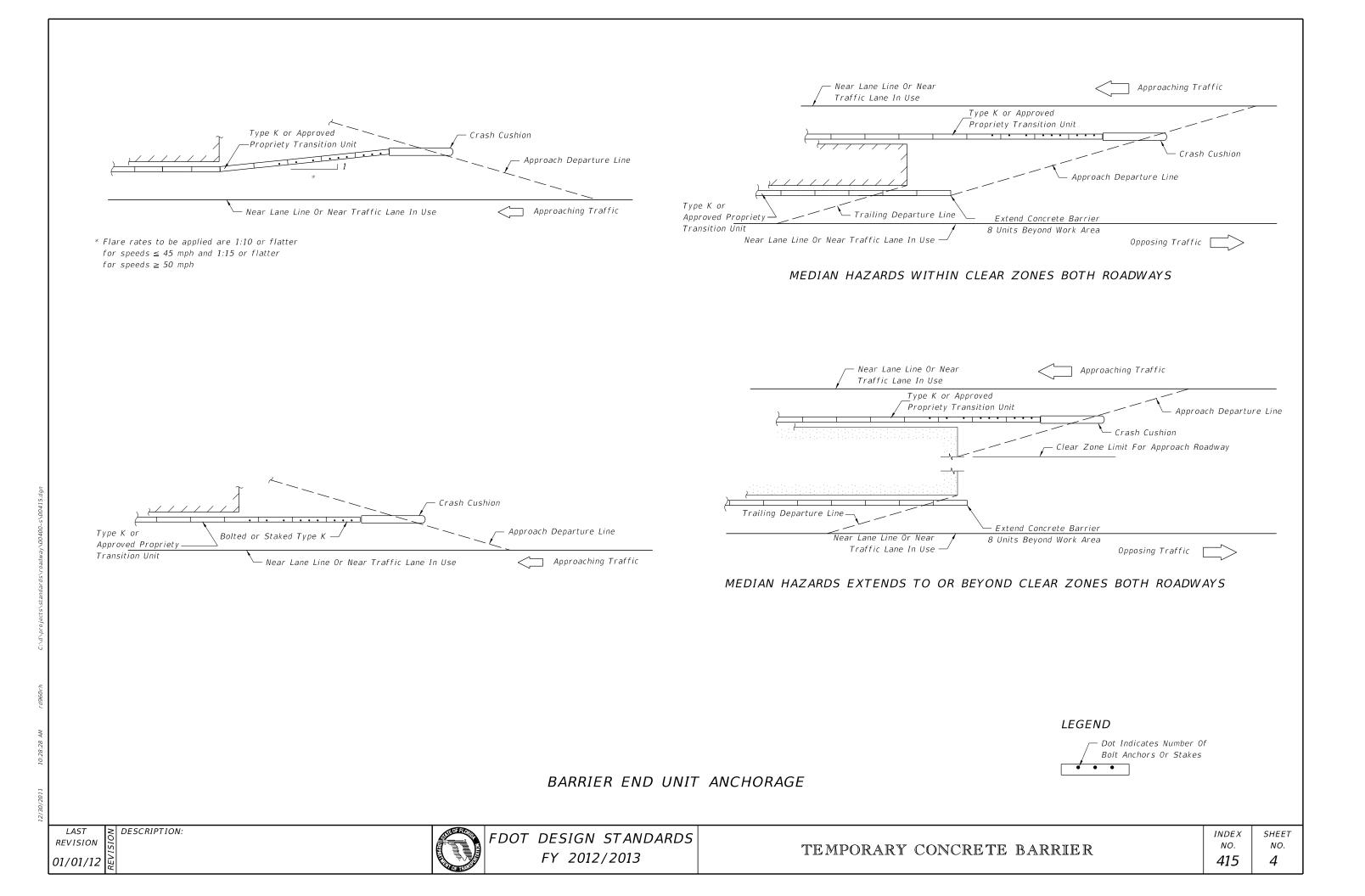


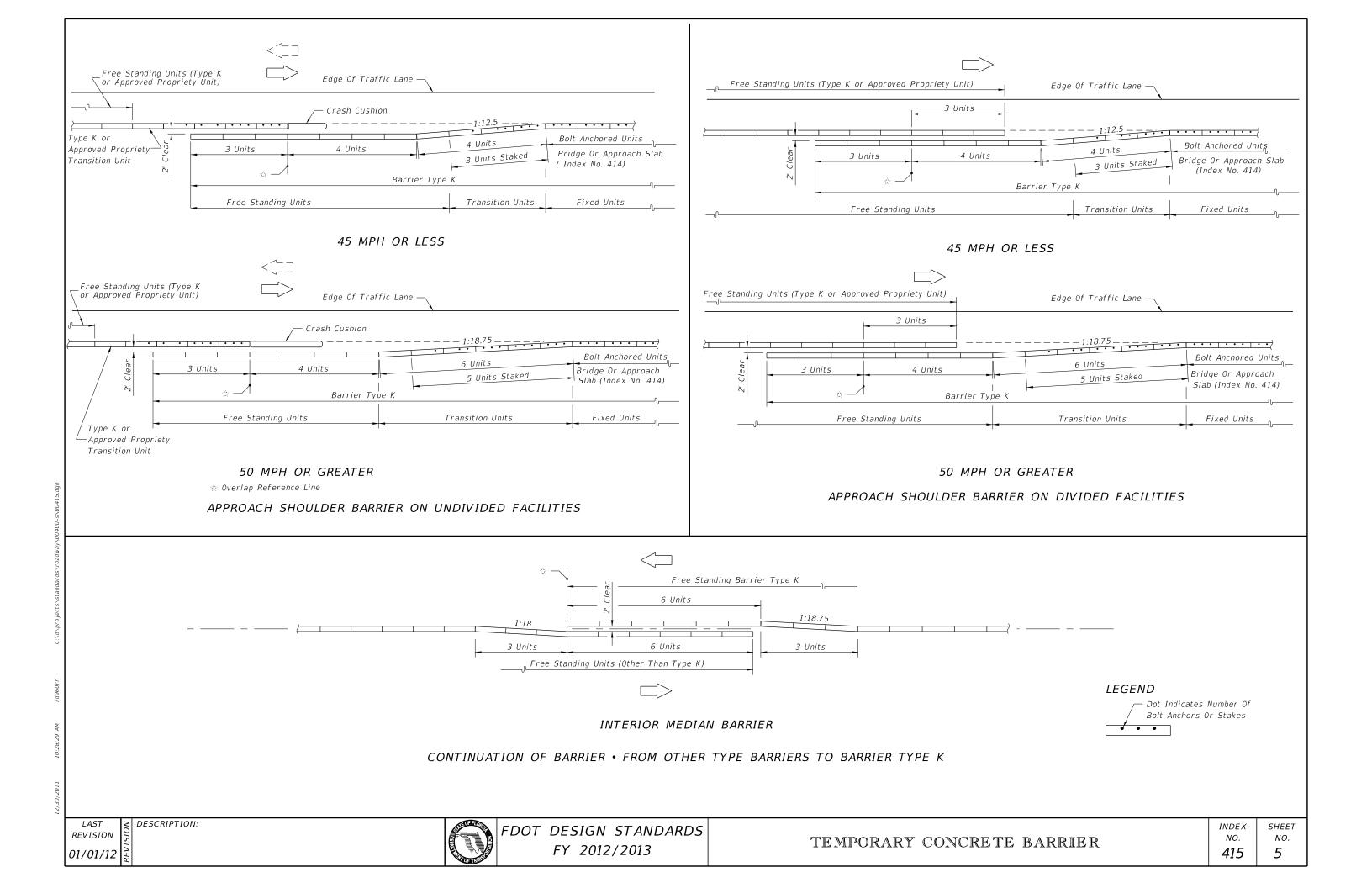


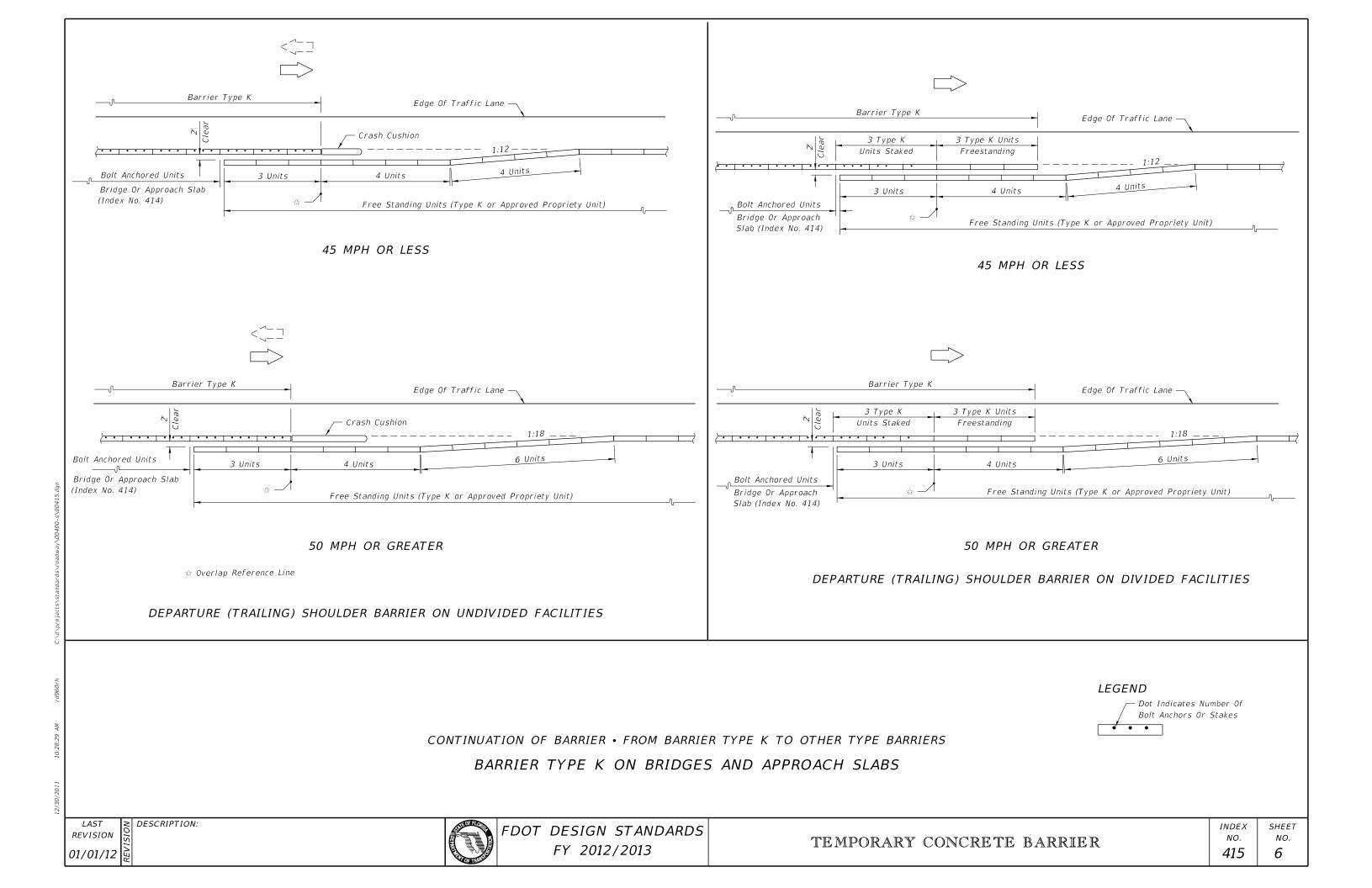
mit — Back Of Hazard Or Back Of Work Area Inside Clear Zone		
Approach Departure Line -	-	
Approach Departure Line	]	
	`~`,	
Approaching Traffic		
Zone Outside Zone Limit		
Clear Zone Limit		
_		
Approach Departure Line		
	~	
Approaching Traffic		
✓ Departure Rates 1:16 For Speeds ≤ 45 mph		
1:13 For Speeds $\geq$ 50 mph		
♦ Area Shielded When Work Zone Ha Work Area Occupy Space Less Tha Width		
♦ Area Shielded When Work Zone Ha Work Area Extend To Or Beyond C		
☆ Crash Cushion In Absence Of Other Shielding. See △ Notations And Sh For Varied Locations For Wall En Crash Cushions.	eet 5 Throu	•
• • Dot Indicates Number Of Bolt Anch Or Stakes	nor s	
	INDEX	SHEET
E BARRIER	<sub>NO.</sub> 415	NО. <b>2</b>

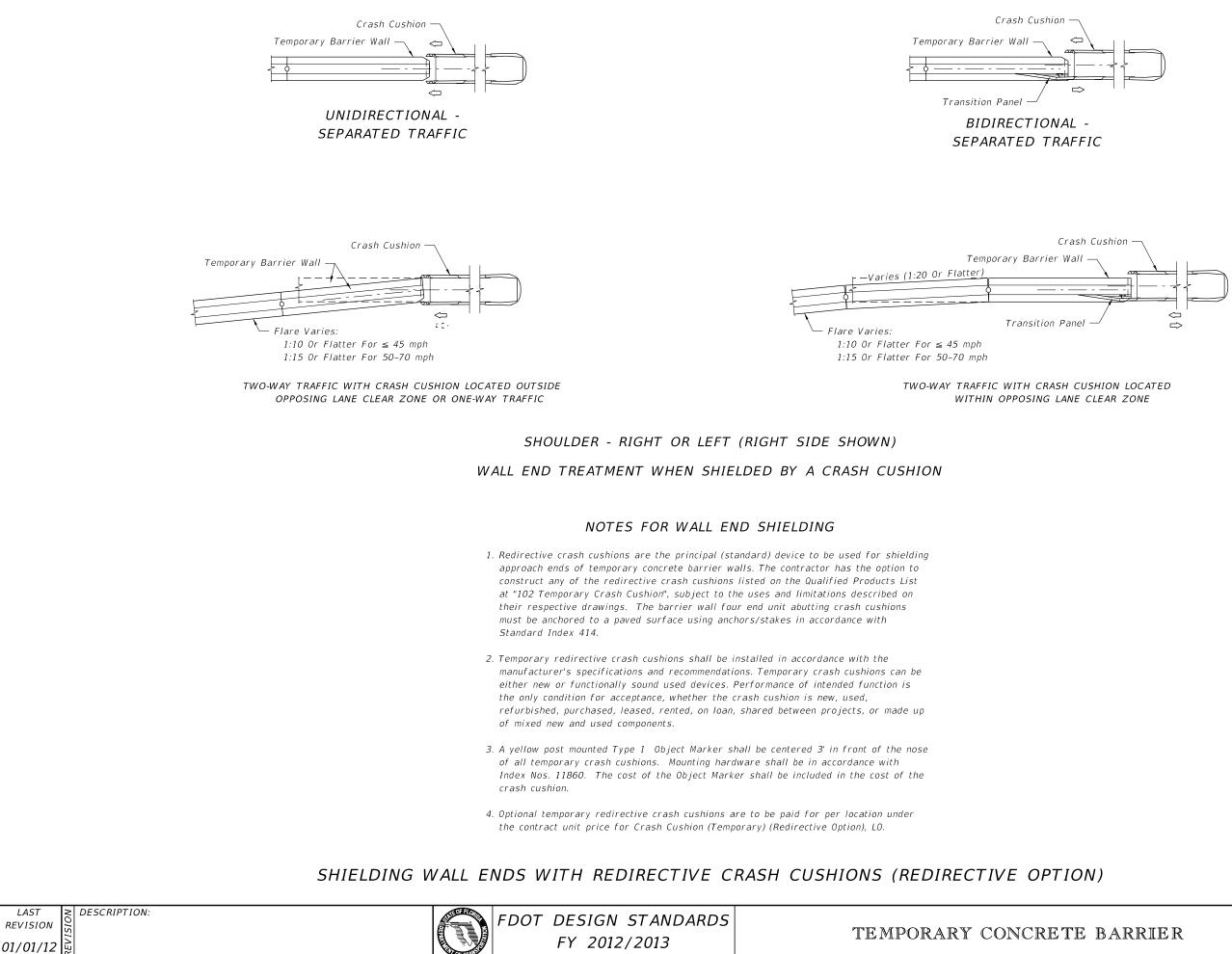


CLEAR ZO	NE WIDTHS FOR I	NORK ZONES
WORK ZONE SPEED (MPH)	TRAVEL LANES & MULTILANE RAMPS (feet)	AUXILIARY LANES & SINGLE LANE RAMPS (feet)
60-70	30	18
55	24	14
45-50	18	10
30-40	14	10
ALL SPEEDS CURB & GUTTER	4' BEHIND FACE OF CURB	4' BEHIND FACE OF CURB



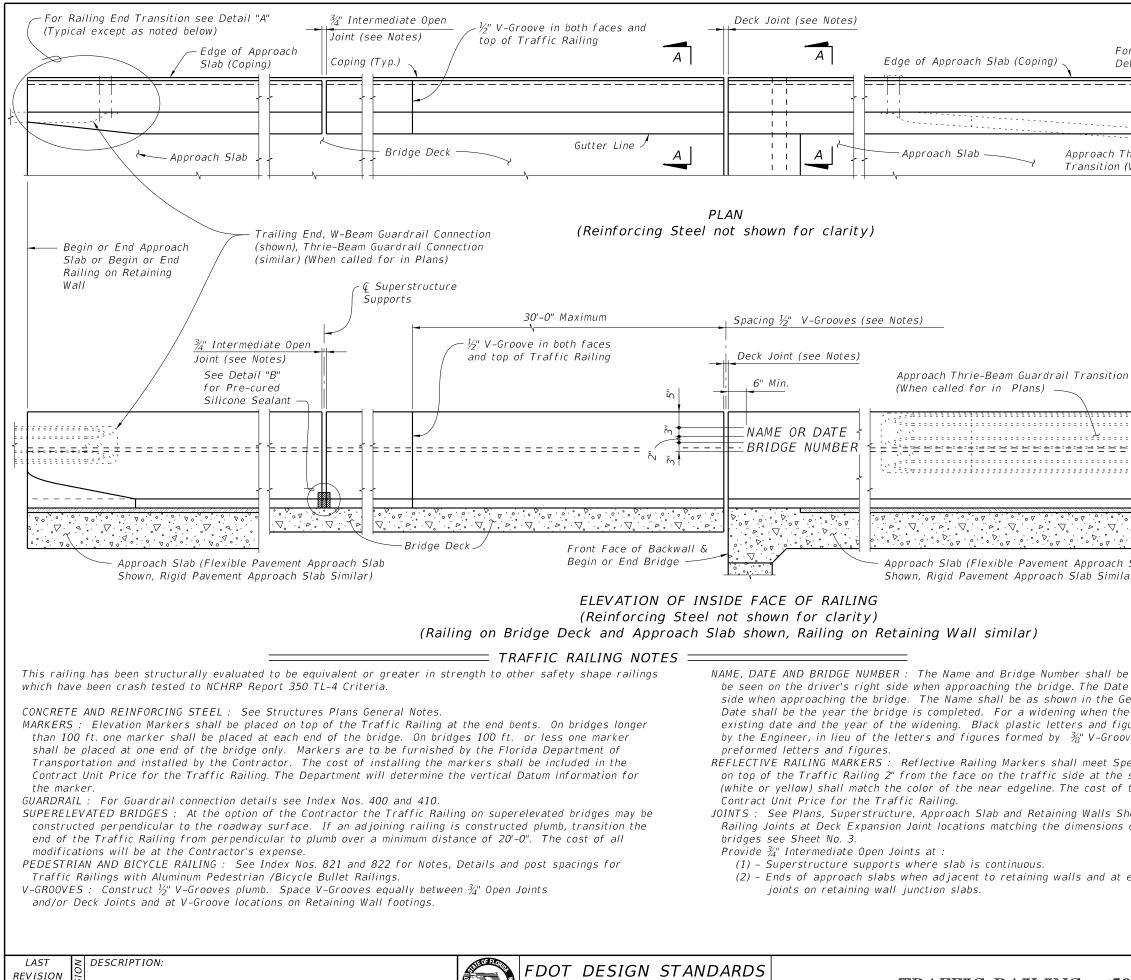






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DN)		
E BARRIER	index no. <b>415</b>	sheet NO. <b>7</b>

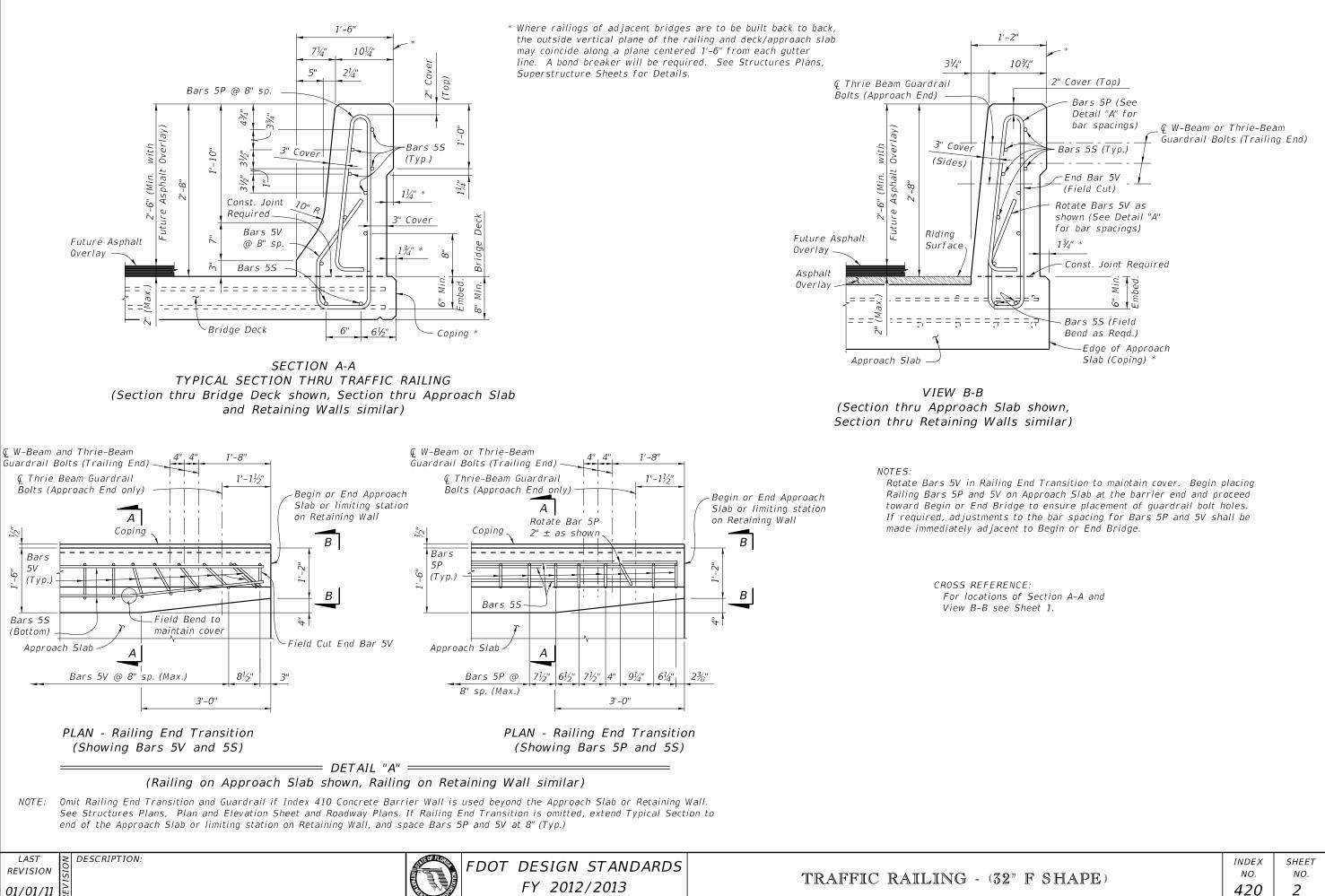


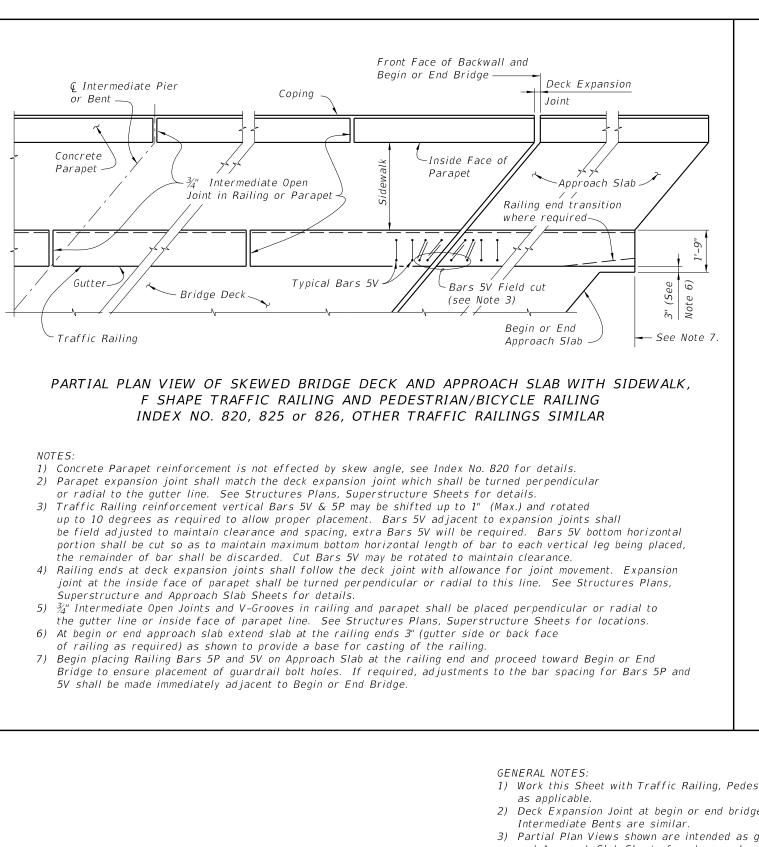
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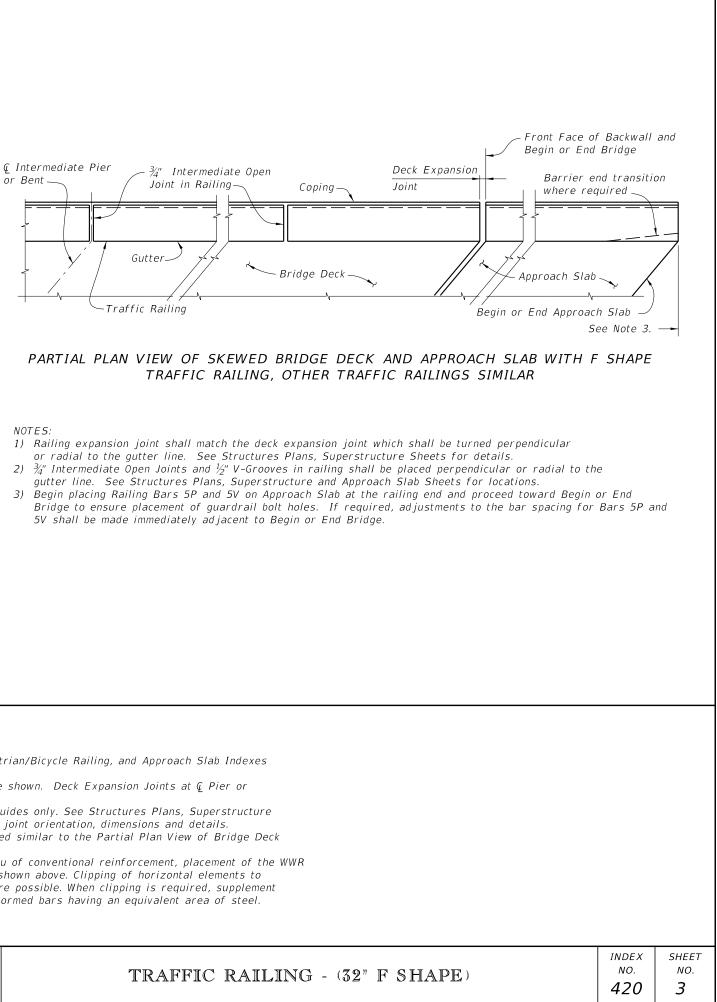
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y	FDOT	FY	2012	/2013

TRAFFIC RAILING - (32

	End Transition see				
etail "A" (T	ypical except as noted)			В	
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1					
	· · · · · · · · · · · · · · · · · · ·	<u> </u>			
hrie-Beam	Guardrail	· · · · ·		B	
When calle	ed for in Plans)				
		<i></i>	- ·		
	Begin or End Approach or End Railing on Reta				
	or Ena nannig on neta	ining wan			
	REFLECTIVE		G		
	MARKER SP	ACING			
	Distance –				
	Edge of Travel Lane	Spacin	g (Ft.)		
	to Face of Railing				
	< 4'	40	ל		
	4' to 8'	80	כי		
	> than 8'	None Re	equired		
			]		
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Slab ar, Typ.) –					
<i>n, ryp.)</i>	CROSS REF	ERENCE:			
			iew B-B aı	nd	
	Detail "A". For Detai				
	FUT Detai	I D, See	Sheet 4.		
placed or	n the Traffic Railing so	as to			
	placed on the driver's le es in the Structures Pla				
	railing is removed, use I				
	height may be used, as				
ves. V-G	rooves shall be formed b	ру			
ecification	Section 993. Install ma	arkers			
spacing sl	hown in the table above.	Reflector			
the reflec	tive markers shall be in	cluded in	the		
neets for a	actual dimensions and jo	int orient	ation. Prov	vide open	
	ck Joint. For treatment				
expansion					
				_	
	<b>ти а ирил</b> а		INDEX NO.	SHEET NO.	
2" F S]	HAPE)				
			420	1	





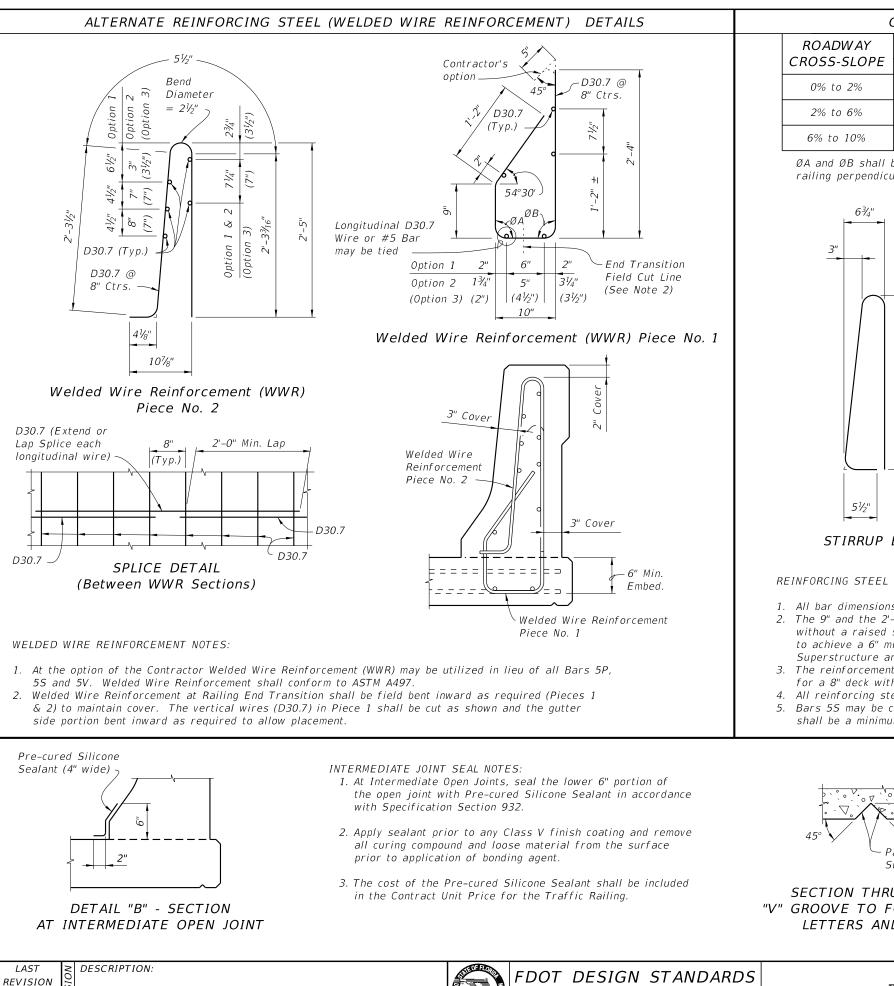


- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at Q Pier or
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing.
- 5) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. When clipping is required, supplement horizontal elements by lap splicing with deformed bars having an equivalent area of steel.

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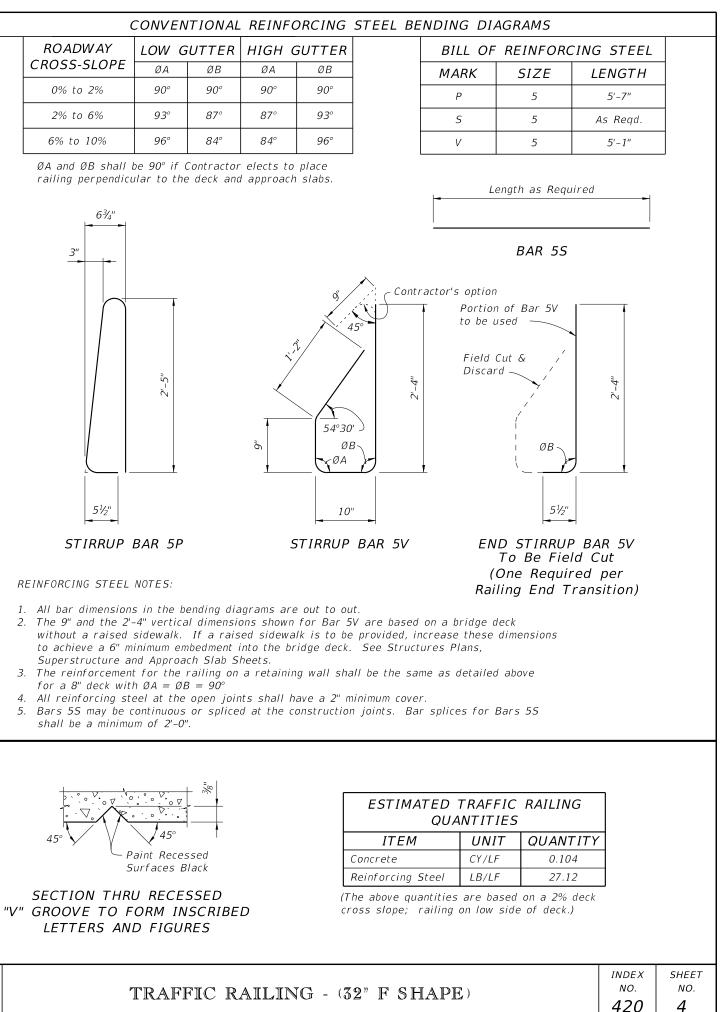
FY 2012/2013



FY 2012/2013

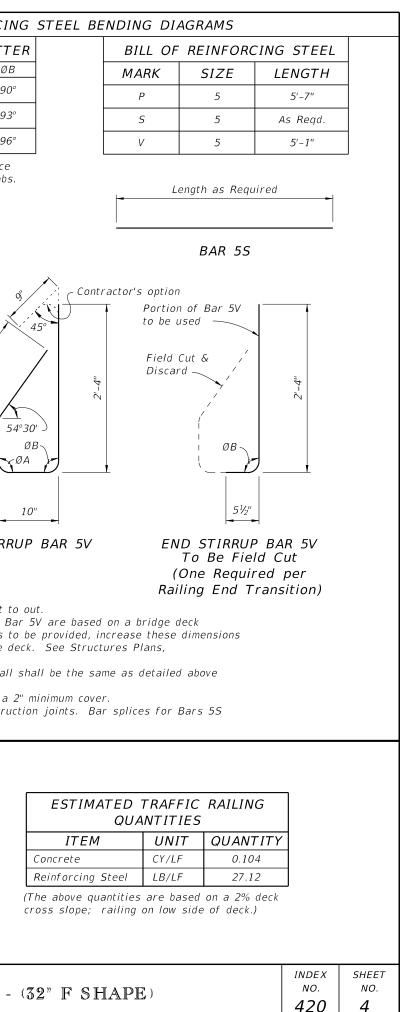
CROSS-SLOPE	ØA	ØB	ØA	ØB
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	93°
6% to 10%	96°	84°	84°	96°
	0.00			

railing perpendicular to the deck and approach slabs.

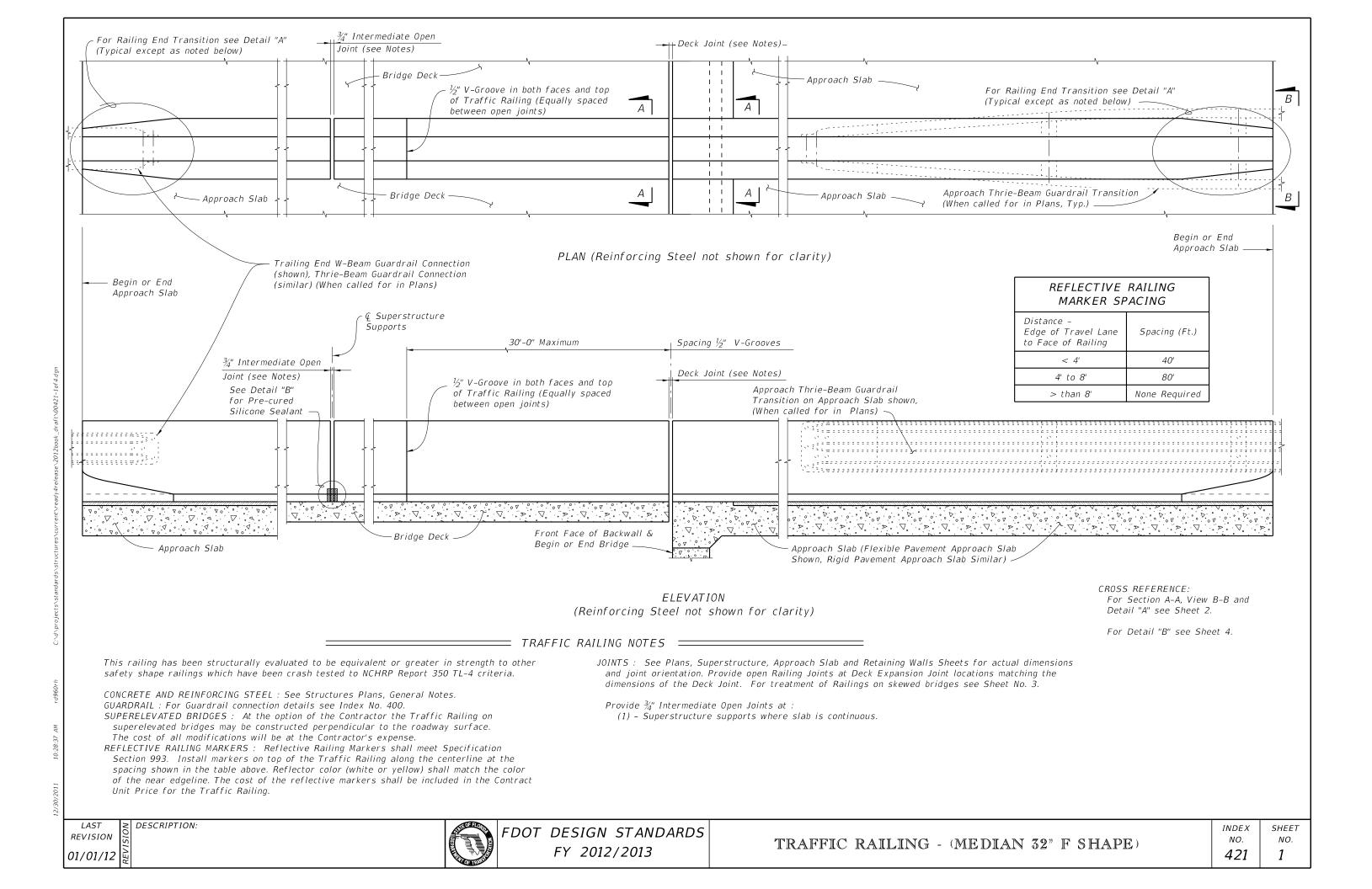


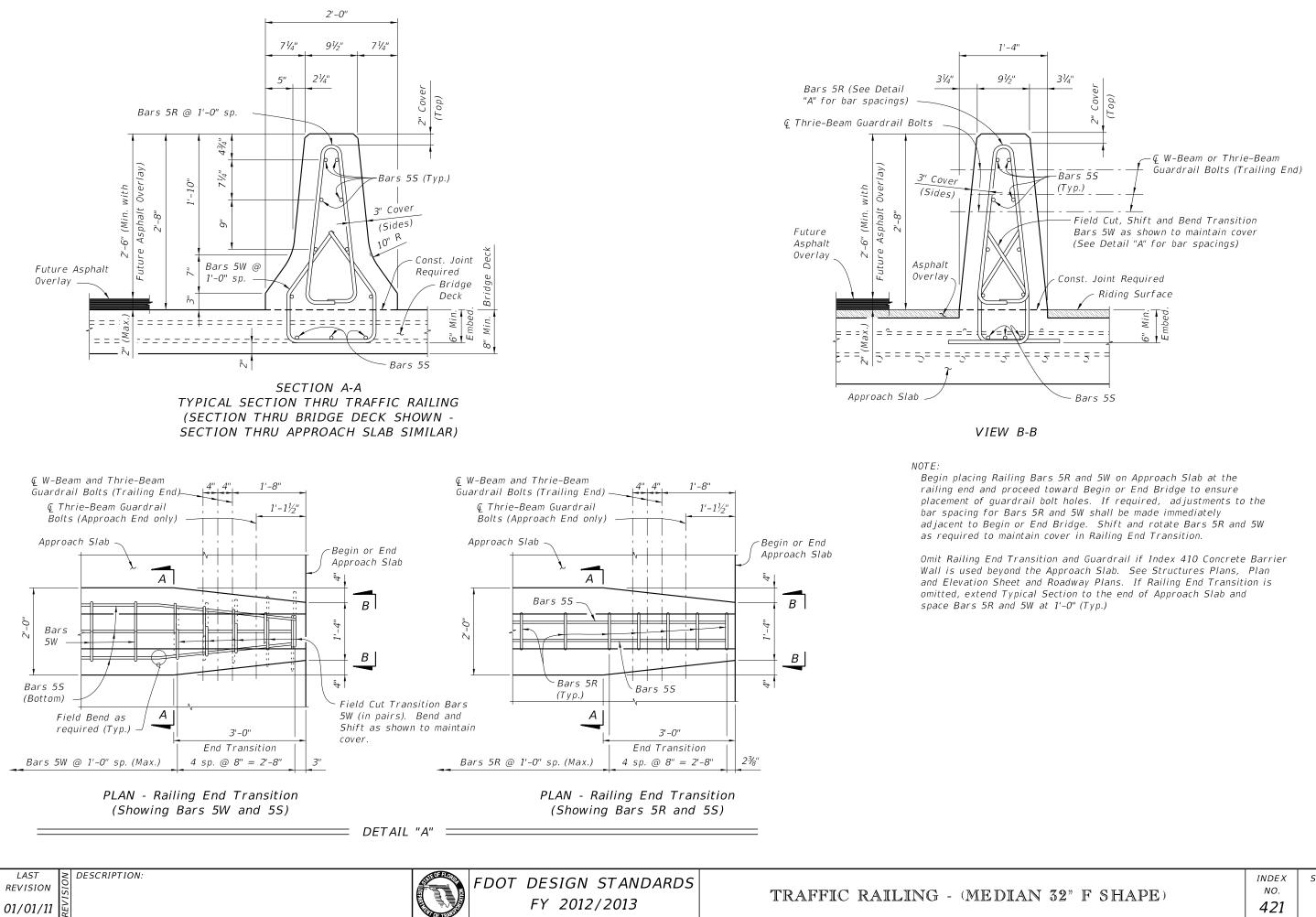
## REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- Superstructure and Approach Slab Sheets.
- for a 8" deck with  $\emptyset A = \emptyset B = 90^{\circ}$
- shall be a minimum of 2'-0".

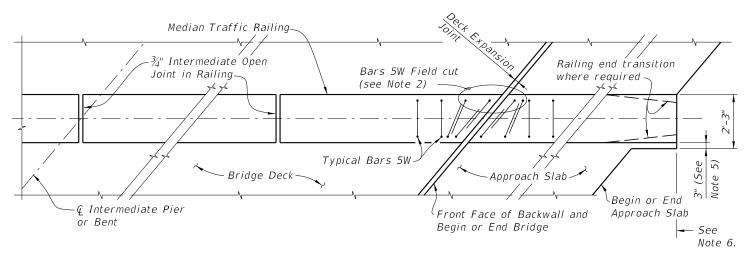


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	INDEX	SHEET
AN 32" F SHAPE)	мо. <b>421</b>	NО. <b>2</b>



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING

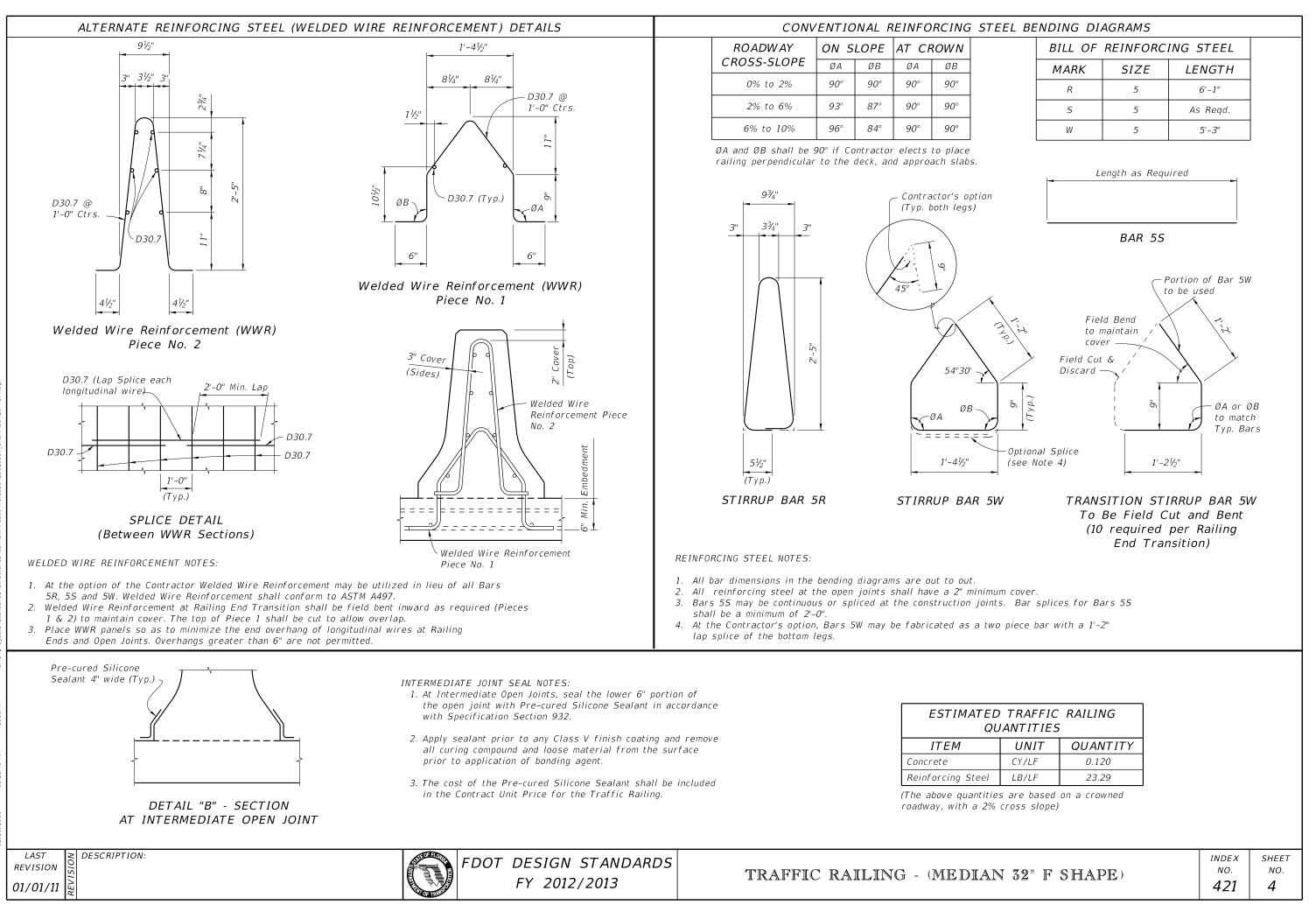
### NOTES:

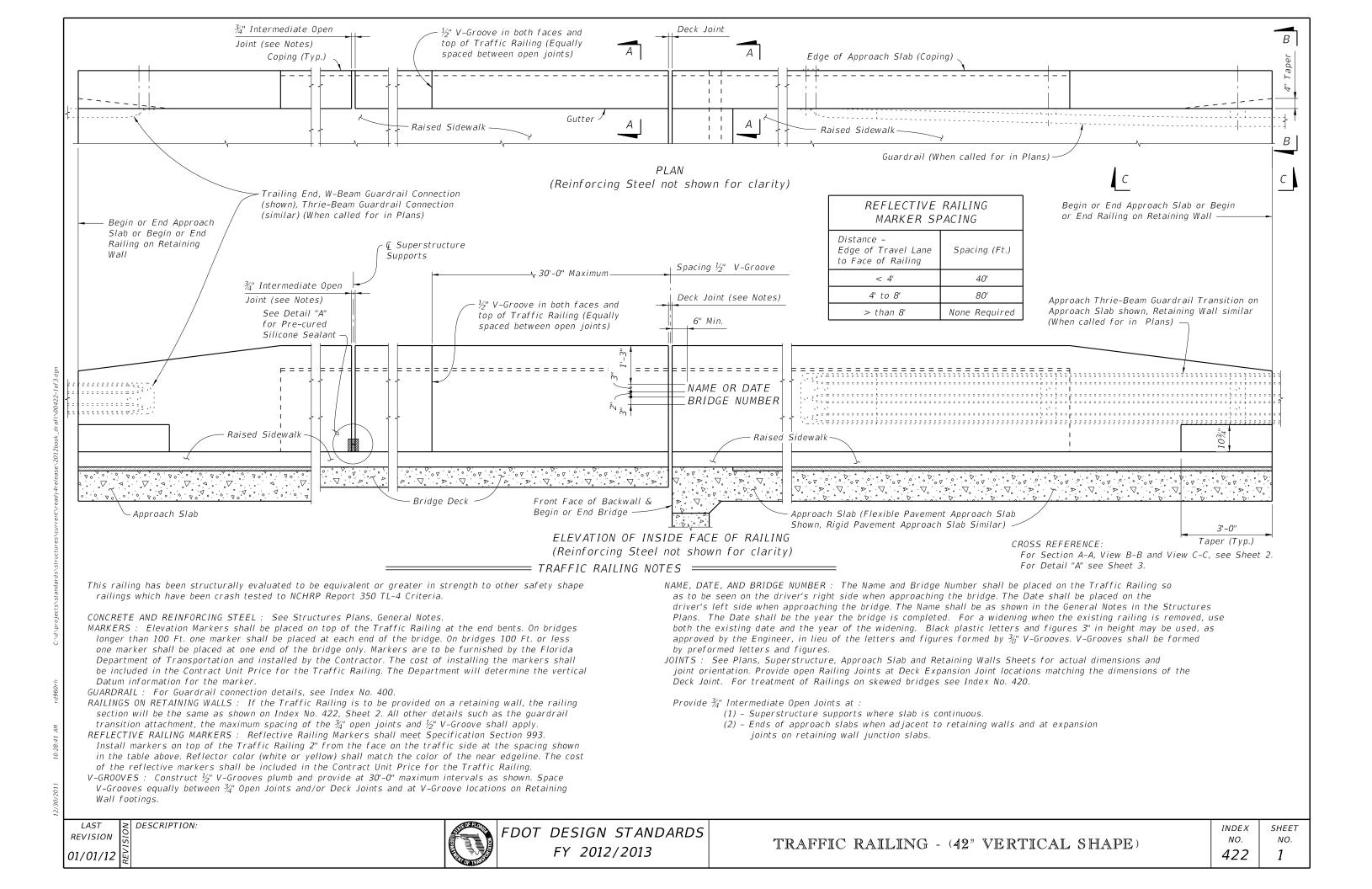
- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4)  $\frac{3}{4}''$  Intermediate Open Joints and V-Grooves in railing shall be placed perpendicular or radial to the Q of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Work this Sheet with Approach Slab Indexes as applicable.
- 7) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at *Q* Pier or Intermediate Bents are similar.
- 8) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 9) If Welded Wire Reinforcement is used in lieu of conventional reinforcement, placement of the WWR vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible. Where clipping is required, supplement horizontal elements by lap splicing deformed bars with an equivalent area of steel.

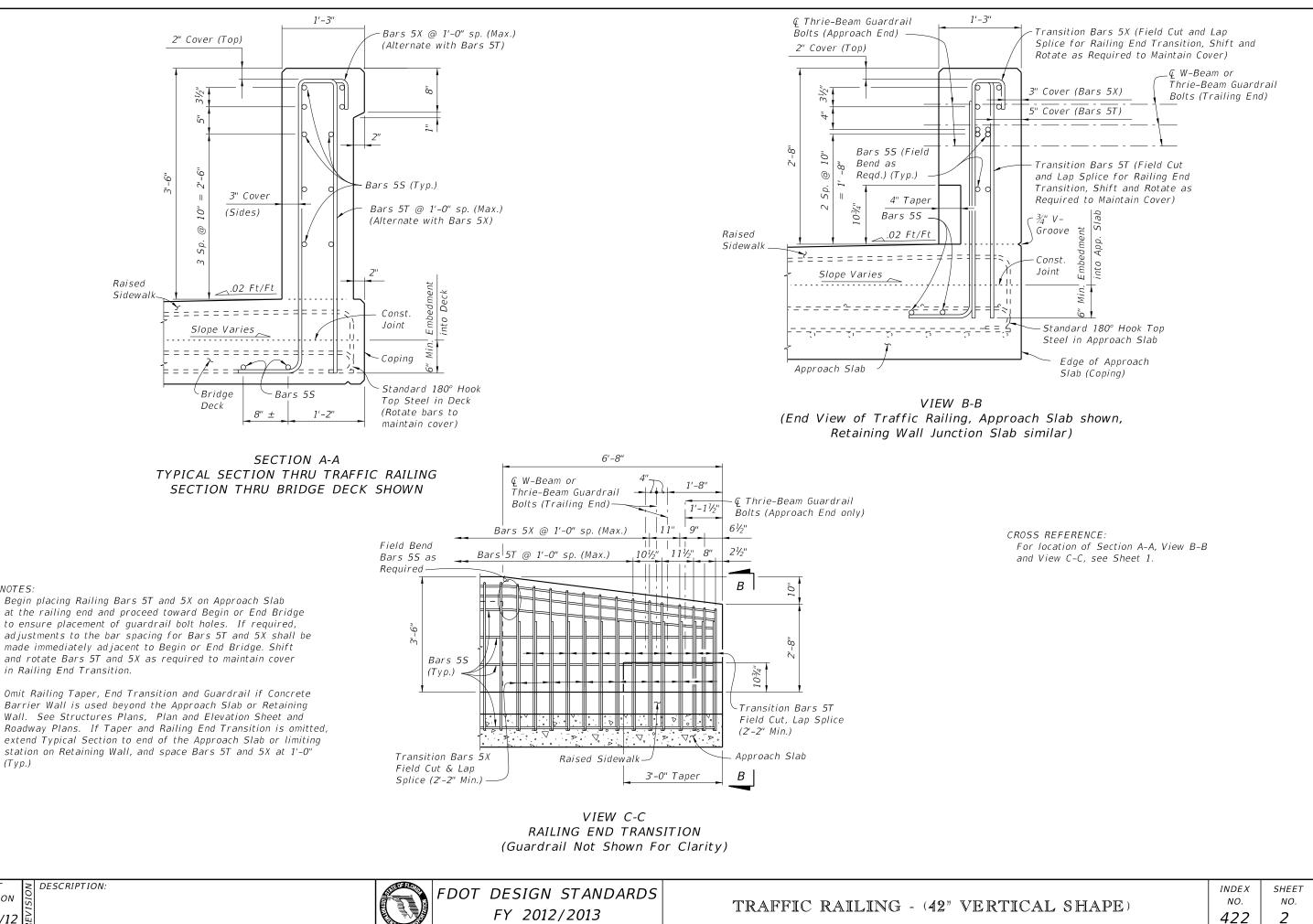
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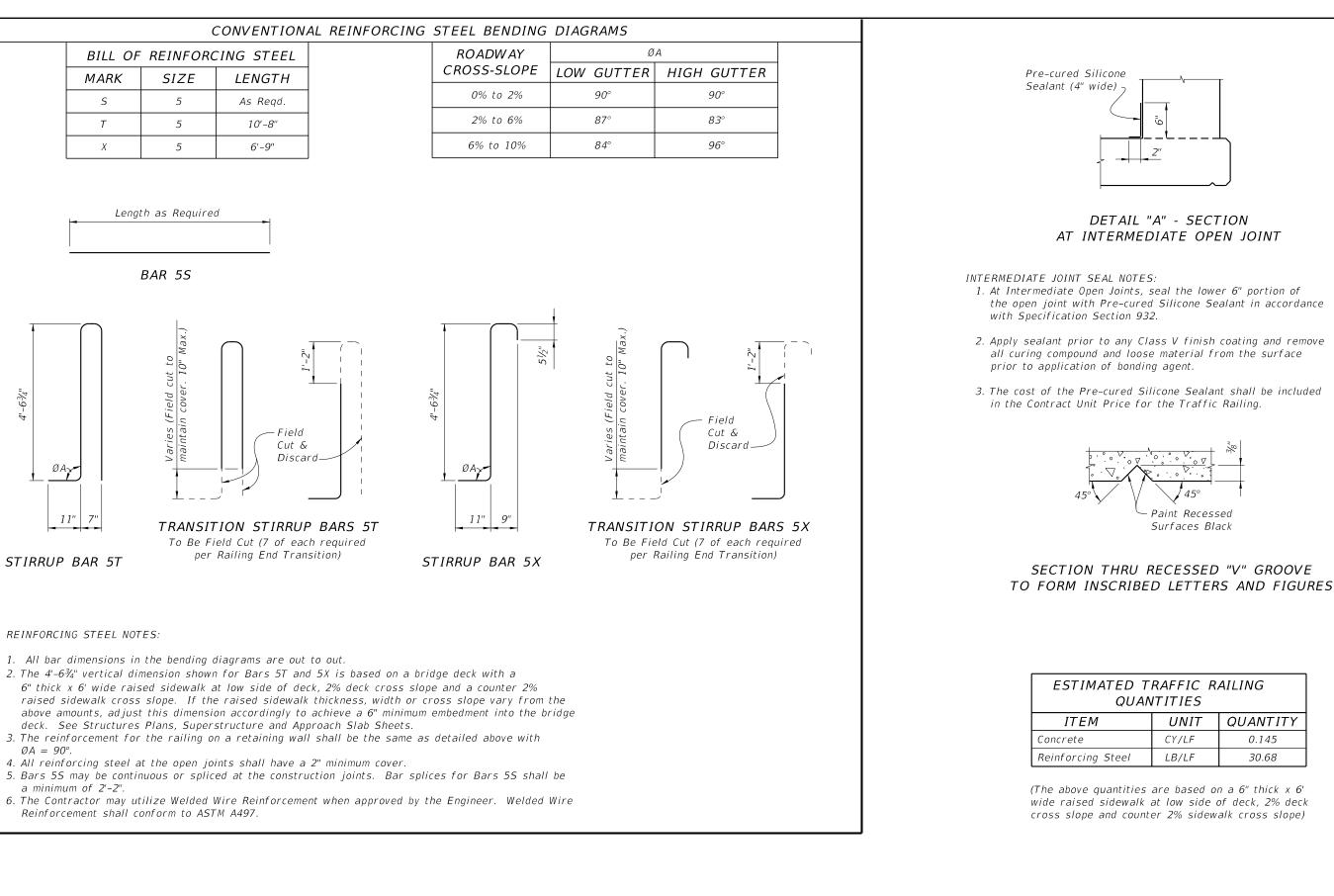






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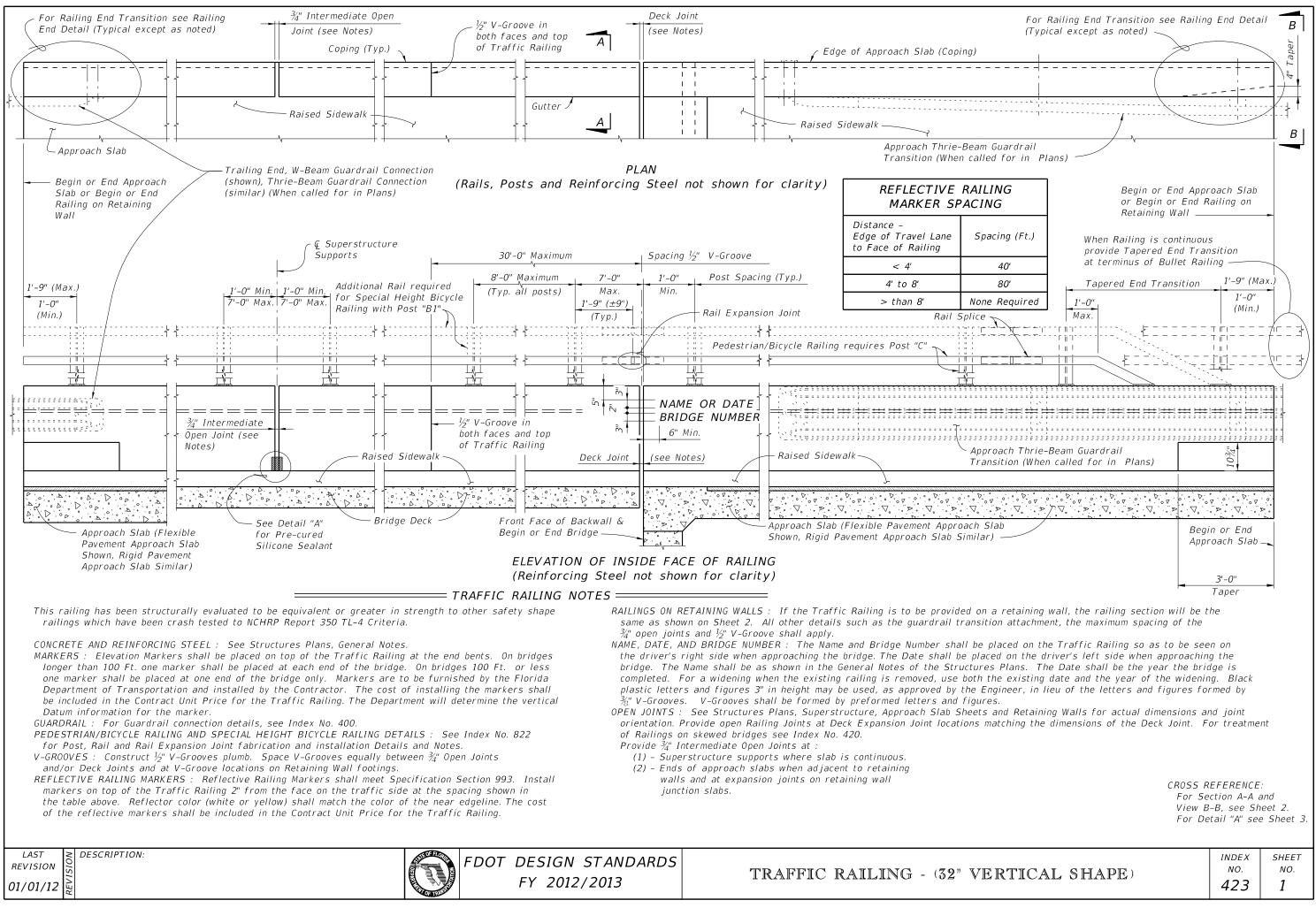
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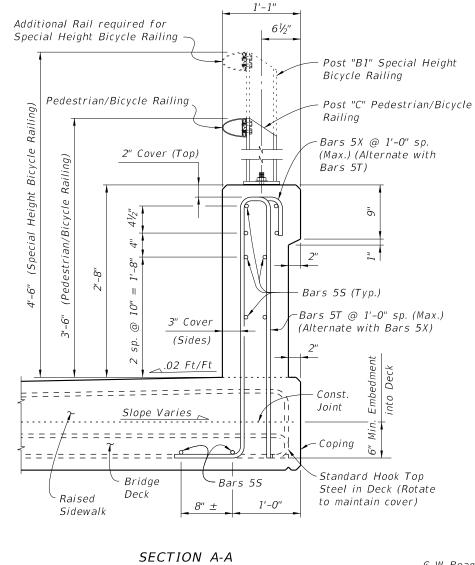
FDOT DESIGN STANDARDS FY 2012/2013

TRAFFIC RAILING - (42" VEF

ATED TRAFFIC RAILING QUANTITIES				
	UNIT	QUANTITY		
	CY/LF	0.145		
Steel	LB/LF	30.68		

$\begin{array}{c} \text{RTICAL SHAPE} \\ \text{A22} \\ 3 \end{array}$	

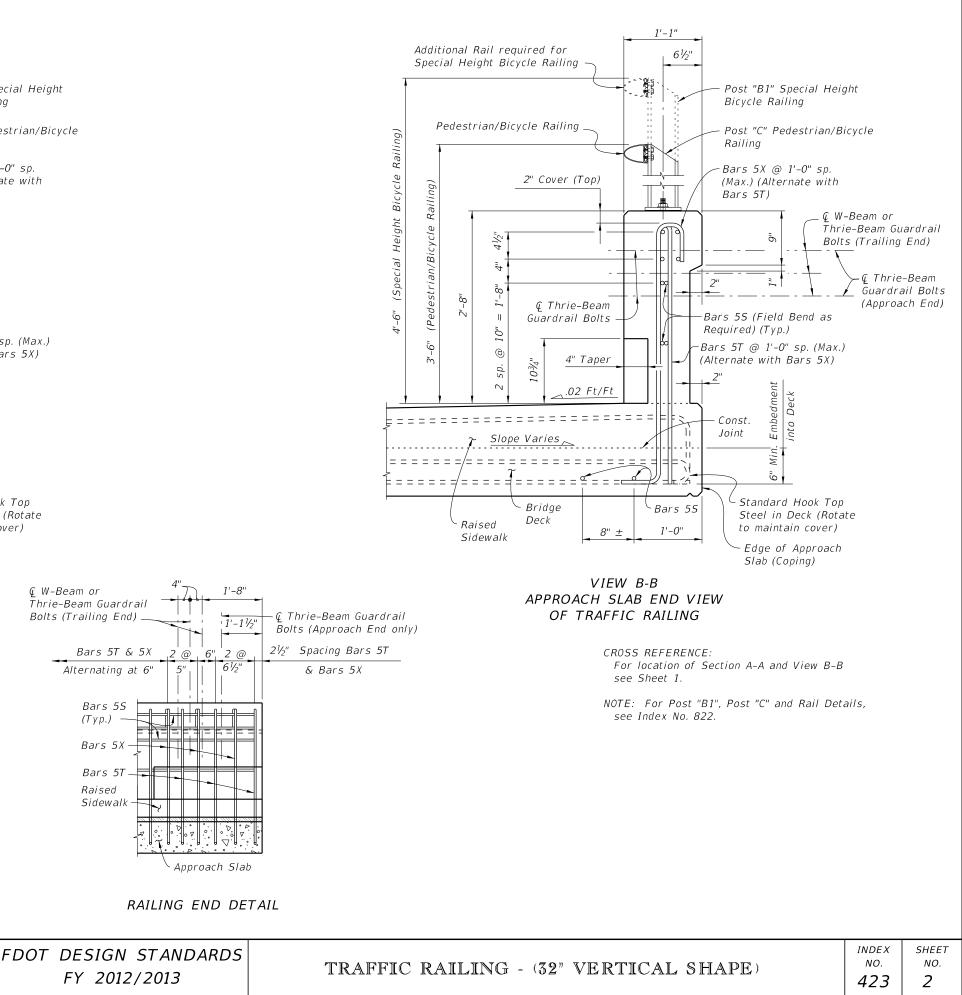






## NOTES:

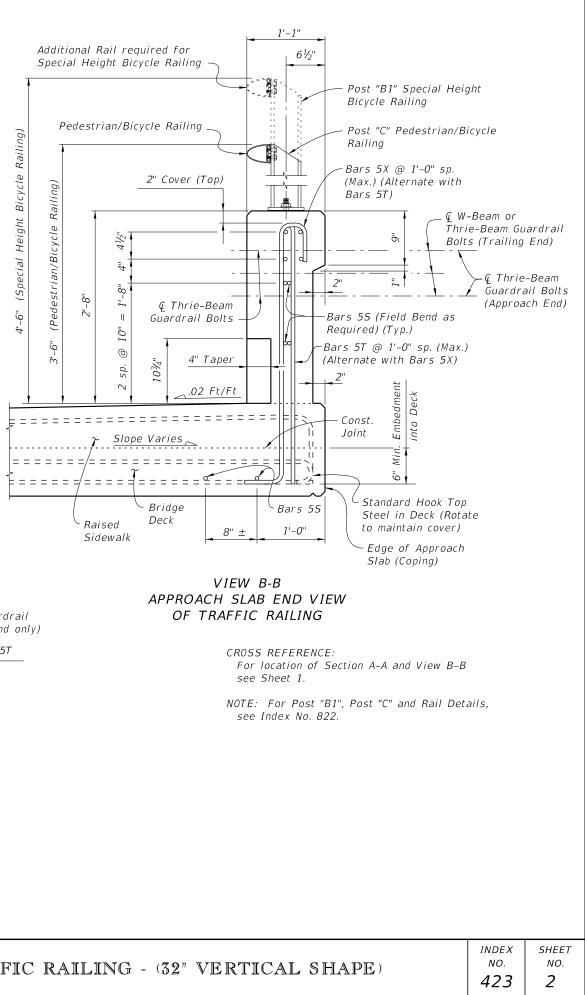
Omit Railing End Taper and Guardrail if Concrete Barrier Wall is used beyond the Approach Slab. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Taper is omitted, extend Typical Section to the end of the Approach Slab. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5T and 5X on Approach Slab in end taper section as required to maintain cover.

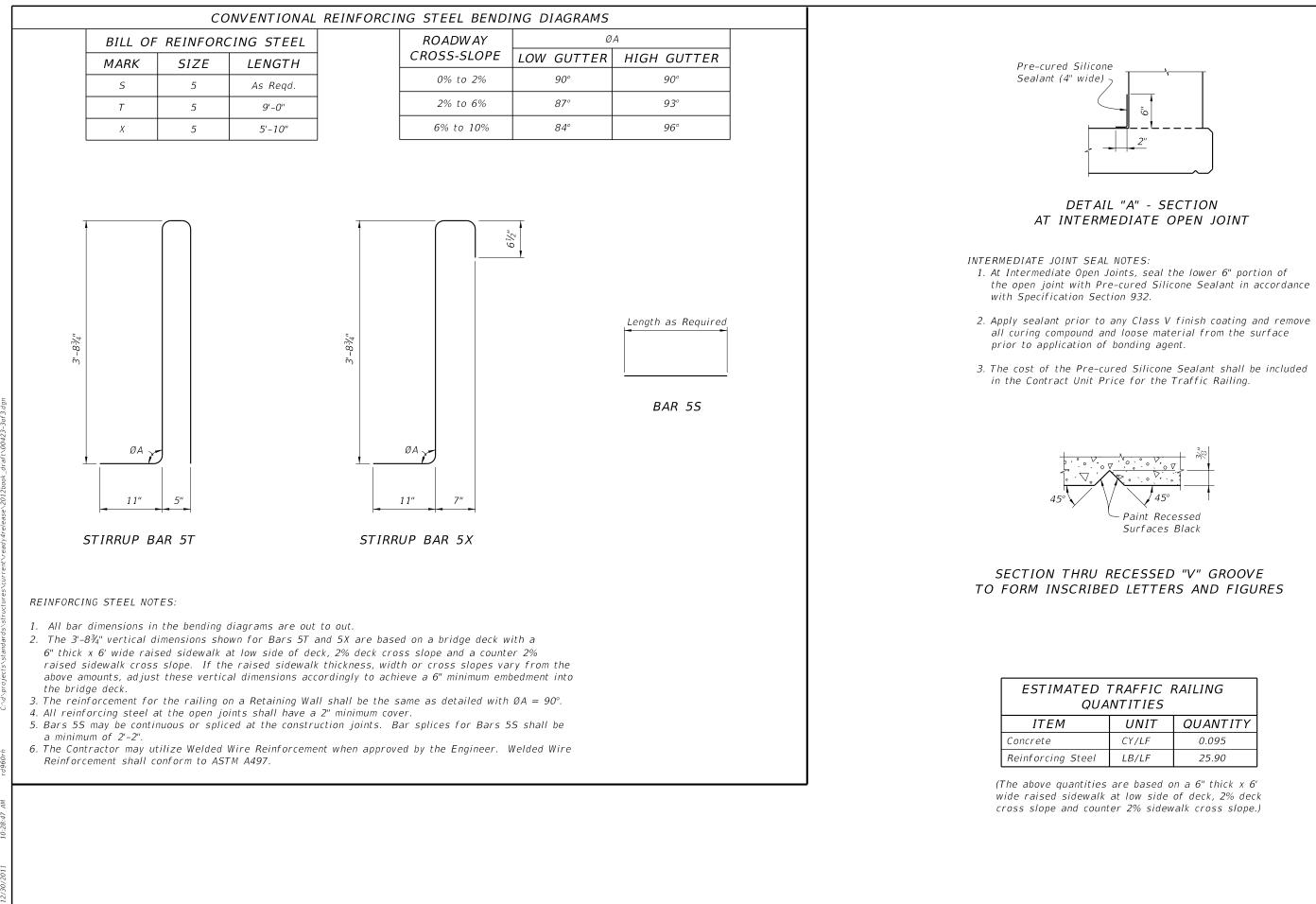


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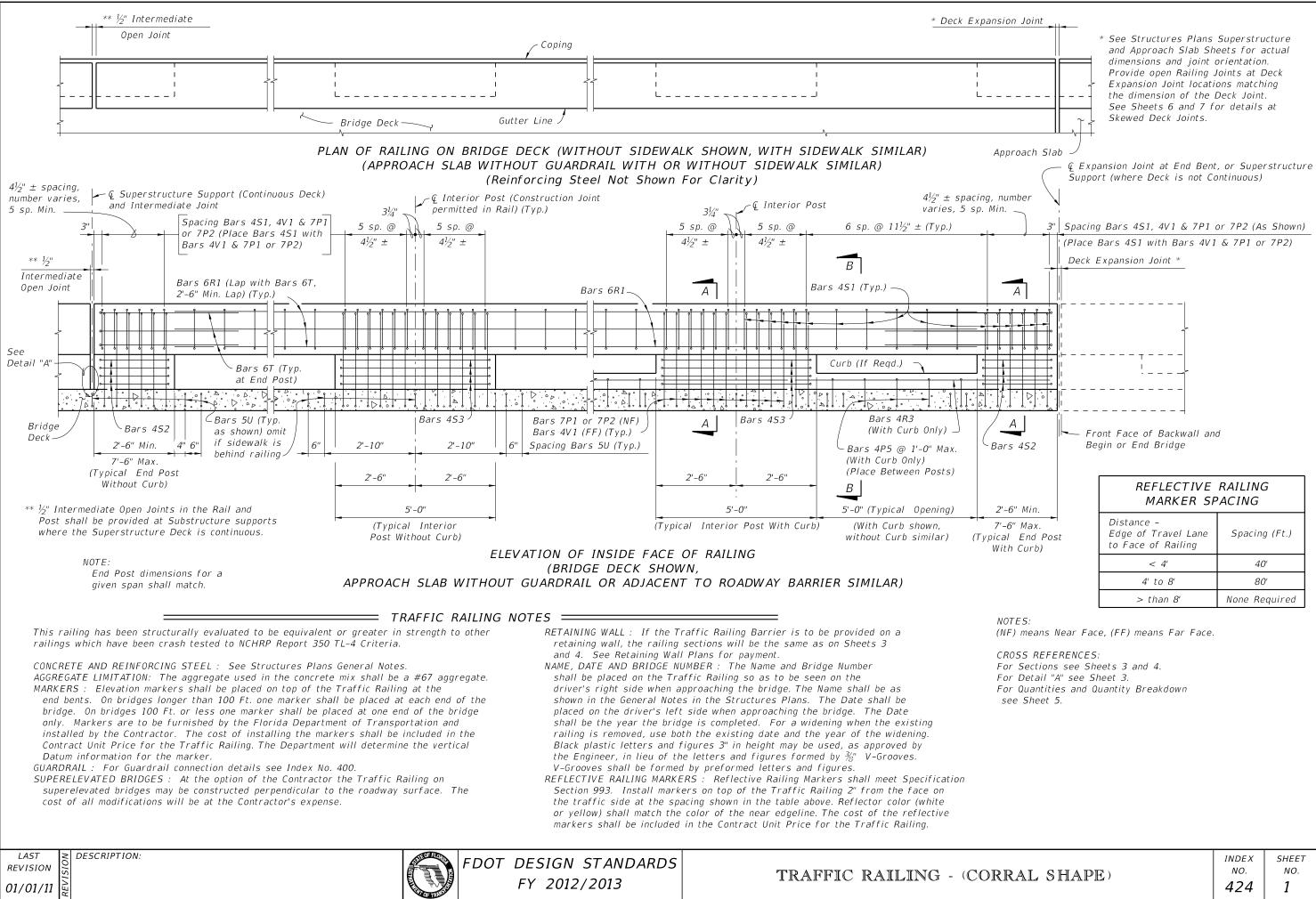
FDOT DESIGN STANDARDS FY 2012/2013

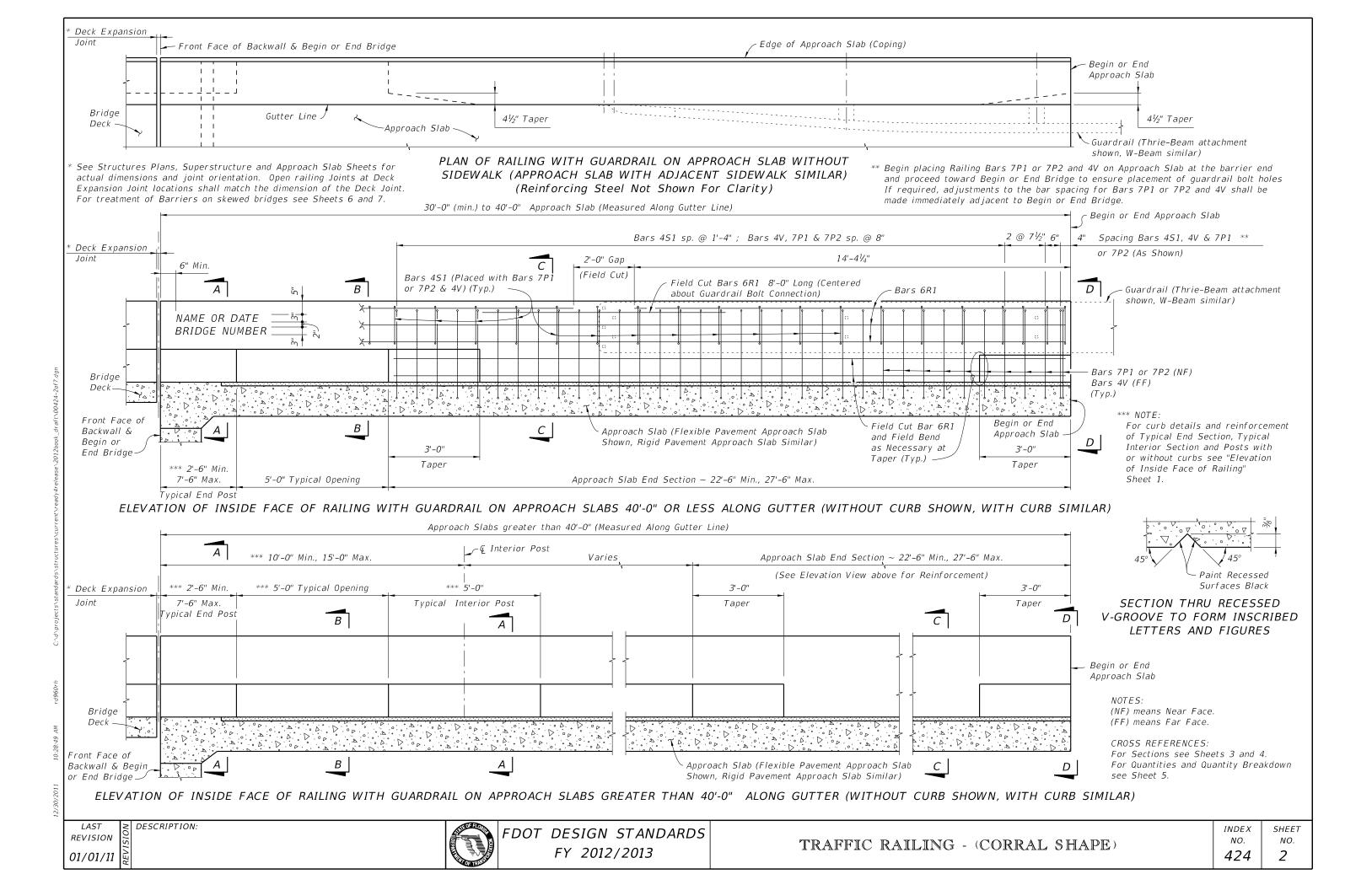
TRAFFIC RAILING - (32" VE

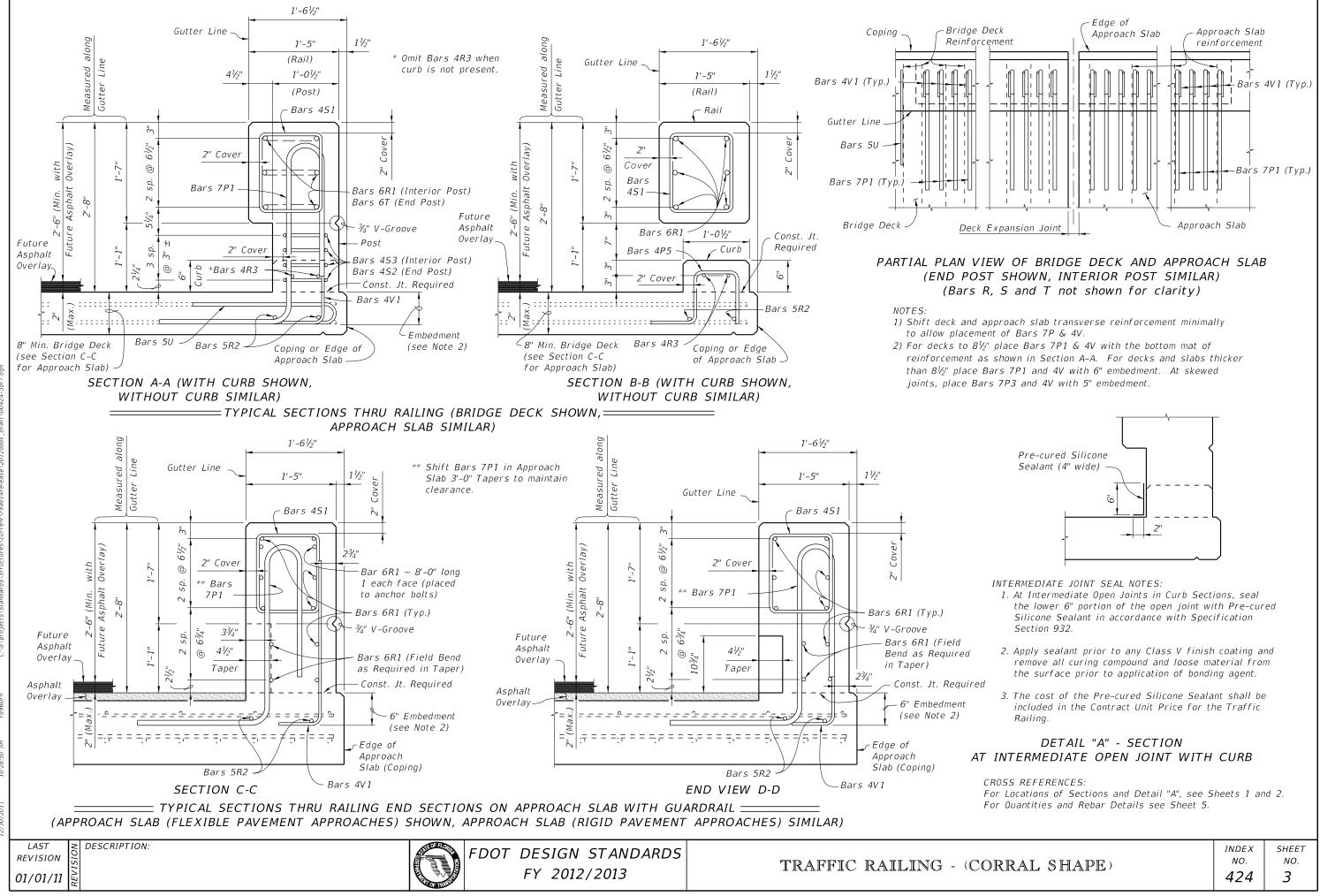
the open joint with Pre-cured Silicone Sealant in accordance

TED TRAFFIC QUANTITIES		RAILING
UNIT		QUANTITY
	CY/LF	0.095
Steel	LB/LF	25.90

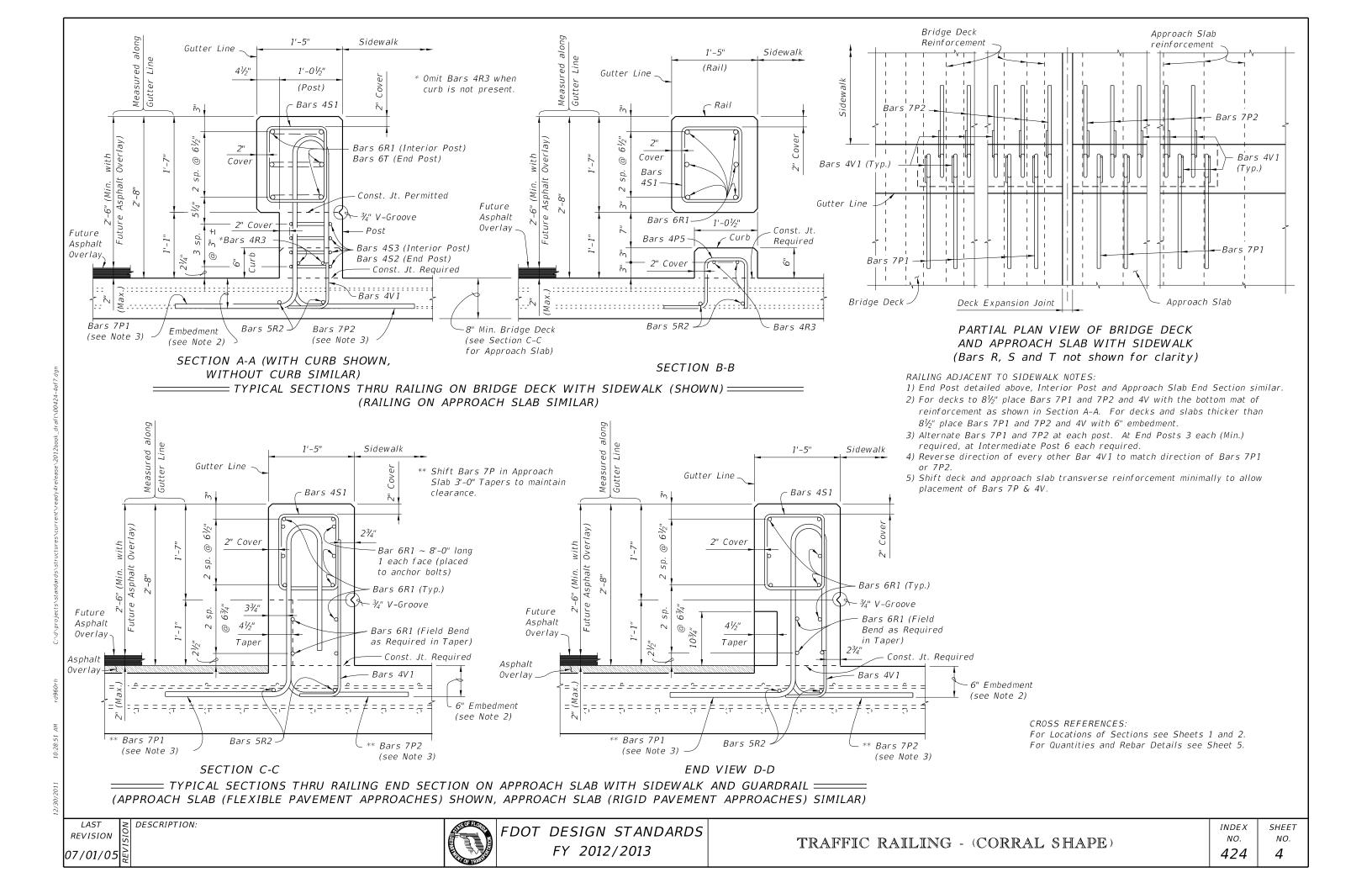
	INDEX	SHEET
RTICAL SHAPE)	NO.	NO.
	423	3



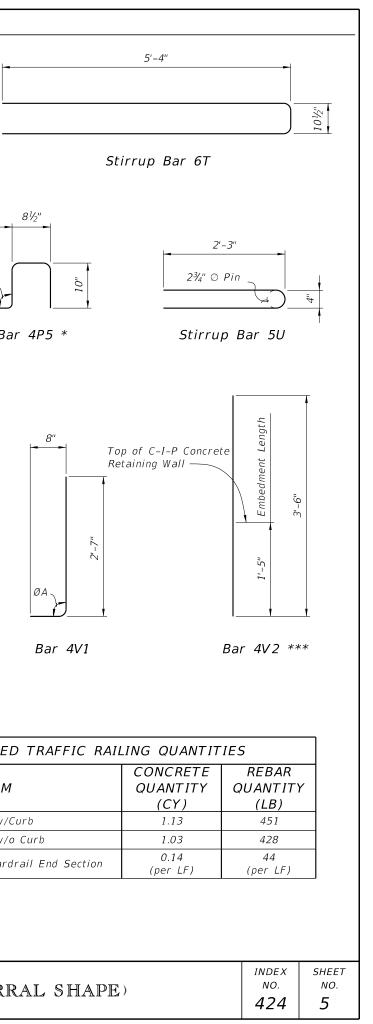


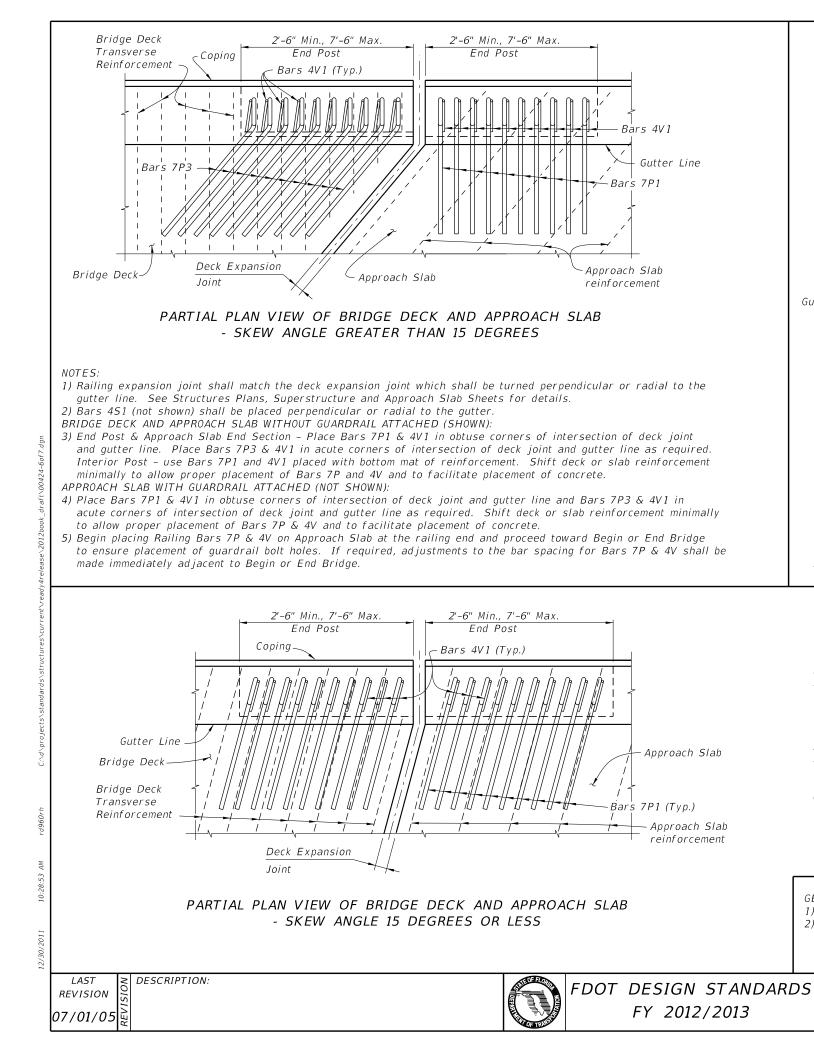


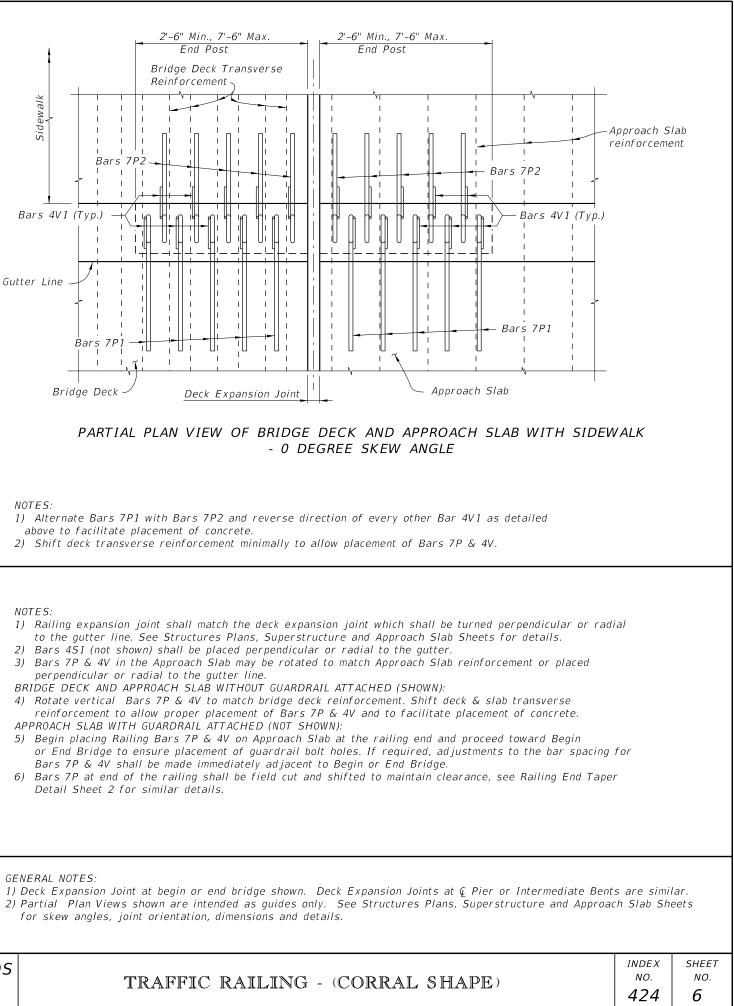
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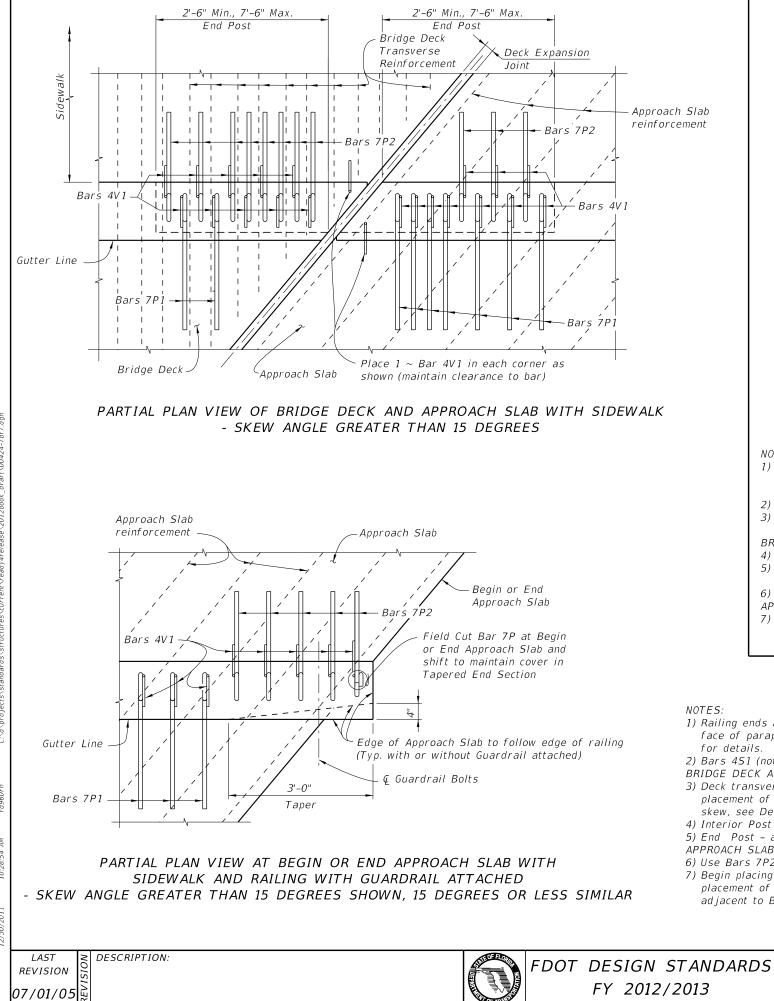


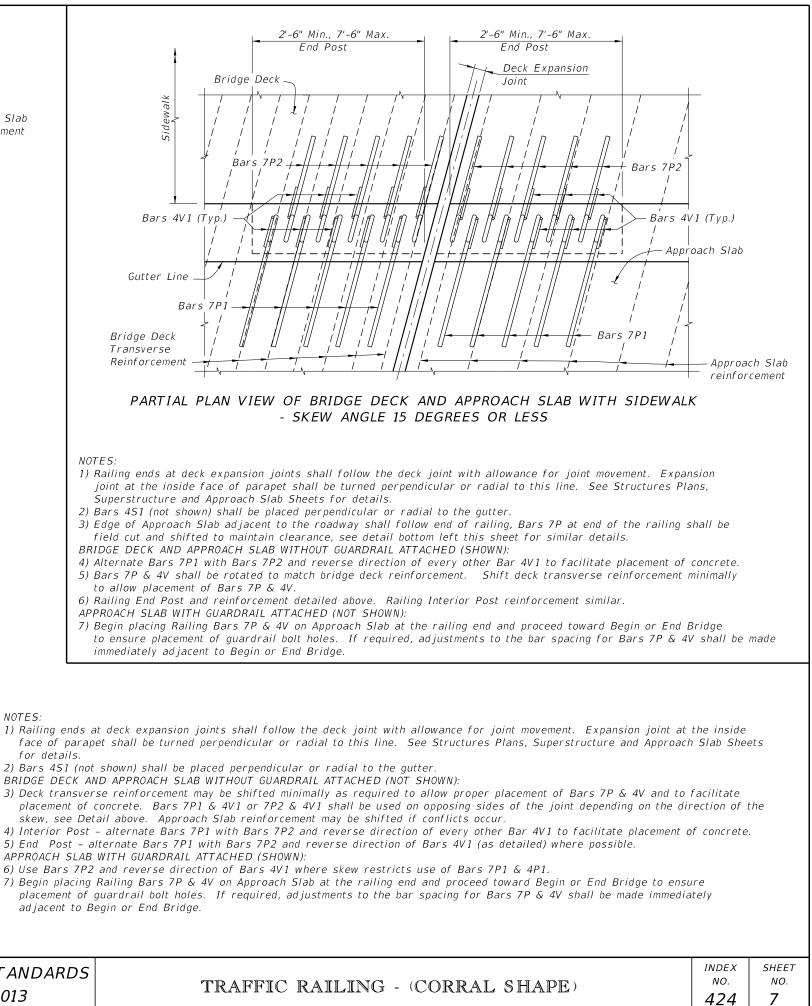
<ul> <li>A. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2"-6" Min. Iap length for Bars 4R3.</li> <li>The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.</li> <li>Description:</li> </ul>					Γ	CONVENT	IONAL REINFOR	CING STEEL	BENDING DIAG	GRAMS	
$\frac{RAR}{2} \frac{R}{r^{2}} \frac{7}{r} \frac{7}{r^{2}} \frac{7}{r^{2}} \frac{1}{r^{2}} \frac{1}{r^{2}$	BILL OI	F REIN	FORCING	STEEL		1'_0"	8½"	-		4'-8"	4
1       1	MARK	SIZE	LENGTH	LB/BAR							
Image: Provide state       Image: Provide state <th< td=""><td>Ρ1</td><td>7</td><td>7'-4''</td><td>15.00</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>	Ρ1	7	7'-4''	15.00							
1       1	Ρ2	7	7'-3"	14.82							81/2
Print       Print <th< td=""><td>Р3</td><td>7</td><td>7'-2"</td><td>14.65</td><td>Length As Required</td><td>1,-3</td><td></td><td><math>\leq \langle \mathcal{N} \rangle</math></td><td></td><td></td><td>· •</td></th<>	Р3	7	7'-2"	14.65	Length As Required	1,-3		$\leq \langle \mathcal{N} \rangle$			· •
$\frac{RT}{12} \frac{6}{2} \frac{k \cdot k \cdot k \cdot k}{12 \cdot k \cdot k \cdot k \cdot k} \frac{12 \cdot k \cdot k \cdot k \cdot k}{12 \cdot k \cdot $	*** P4	7	7'-3"	14.82				<u>- S5 ~</u> 7'-2"	Stir	rup Bar 4S3	
$\frac{11}{128} \frac{1}{2} \frac{1}{4} \frac$	* P5	4	2'-11''	1.94				Varie			
$\frac{1}{4} \frac{1}{1} \frac{1}{4} \frac{1}$	R1	6	As Reqd.	1.5 (LB/LF)	Bars 6R1, 5R2 & 4R3	Stirrup Bar 4S1	$\bigcup_{i=1}^{n}$	<u> </u>			8"
All of the state is described by the state is in the stat	R2	5	As Reqd.	1.04 (LB/LF)			Stirrup Par	157			
$\frac{1}{10000000000000000000000000000000000$	* R3	4	As Reqd.	0.67 (LB/LF)			Зсптир Ба	432			
$\frac{1}{10} \frac{1}{10} \frac$	** 51	4	5'-0''	3.34						2'-1	ØA
$\frac{1}{10} \frac{1}{10} \frac$	** 52	4	6'-3" Min.	4.18 Min.	2'-1" 7"					5,-1"	<u>}</u> B
$\frac{1}{1000} \frac{1}{1000} \frac{1}{10000} \frac{1}{10000} \frac{1}{10000} \frac{1}{100000} \frac{1}{10000000000000000000000000000000000$	** 53	4	11'-3"	7.52							
$\frac{ \mathbf{v}_{1} }{ \mathbf{v}_{2}  ^{2}} \frac{ \mathbf{v}_{1} }{ \mathbf{v}_{2}  ^{2}} \frac{ \mathbf{v}_{2}  ^{2}}{ \mathbf{v}_{3}  ^{2}} \frac{ \mathbf{v}_{1}  ^{2}}{ \mathbf{v}_{3}  ^{2}} $	Т	6	11'-4''	17.02							
$\frac{1}{2} \frac{1}{2} \frac{1}$	U	5	4'-8''	4.87		=	=			-Z <sup>-1</sup>	
<ul> <li>Bars 4PS and 4R3 are to be used with the bars 4S1, MS2 6 4S3 around a bars 4S2, MS2 6 4S3 around a bars 4S1, MS2 6 4S3 4S1, MS2 6 4S3</li></ul>	V 1	4	3'-2"	2.12	10"	2'-1	2'-1				
<ul> <li>a curb only.</li> <li>a curb only.</li> <li>Berr Berd Burs 451, 452 &amp; 453 around a 2 3 for row Pin.</li> <li>Burs 724 &amp; 472 are to be used on C-1-P Concrete Retaining Walls.</li> <li>Bar 7P1</li> <li>Bar 7P1</li> <li>Bar 7P1</li> <li>Bar 7P1</li> <li>Bar 7P2</li> <li>Bar 7P1</li> <li>Bar 7P2</li> <li>Bar 7P3 (Requires 3 Dimensional Bend)</li> <li>Concrete Applicable. If battom to reaction of the bendrog diagrams are out to out.</li> <li>The reinforcement for the rating on a C-1-P Concrete Retaining Walls.</li> <li>A that origination and 97 any very from joint to joint and side to solve and side to side, see Structures Plans, Superstructure Sheats for details.</li> <li>The seew angle for Bars 7P3 may very from joint to joint and side to side, see Structures Plans, Superstructure Sheats for details.</li> <li>The seew angle for Bars 7P3 may very from joint to joint and side to side, see Structures Plans, Superstructure Sheats for details.</li> <li>PESCRIPTION:</li> </ul>	*** V2	4	3'-6"	2.34	Ň,	Ž -			Parallel to Joint~		
<ul> <li>1. All bar dimensions in the bending diagrams are out to out.</li> <li>2. The reinforcement for the railing on a C-I-P Concrete Relating Wall shall be the same as detailed above for a 8' deck with DA = 90', where applicable. If bottom horizontal legs of Bars 7P1, 7P3 and 4V1 prohibit bacement, Bars 7P4 and 4V2 may be substituted for Bars 7P1, 7P3 and 4V1 as shown.</li> <li>3. All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.</li> <li>4. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2-6" Min. lap length for Bars 5R2 and a 1'-3" Min. lap length for Bars 7R3.</li> <li>5. The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.</li> <li>CACOSS-SLOPE 0.A dA dA dA dA dA dA dV dV</li></ul>					Bar 7P1	ROADWAY OR SIDEWALK	HIGH LOW	Superstruct	Bar 1 3 Dim 7"		
shall be the same as detailed above for a 8" deck with 0A = 90°,         where applicable. If bottom horizontal legs of Bars 7P1, 7P3 and 4V1         prohibit placement, Bars 7P4 and 4V2 may be substituted for Bars 7P1,         7P3 and 4V1 as shown.         3. All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.         4. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2-6° Min. Iap length for Bars 5R2 and a 1'-3" Min. Iap length for Bars 4R3.         5. The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.         4. Mats to side, see Structures Plans, Superstructure Sheets for details.         EXERCIPTION:         EVELOW         EXERCIPTION:         EVELOW         EVELOW      <											
prohibit placement, Bars 7P4 and 4V2 may be substituted for Bars 7P1, 7P3 and 4V1 as show.         3. All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.         4. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2-6" Min. Iap length for Bars 5R2 and a 1'-3" Min. Iap length for Bars 4R3.         5. The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.         LAST       Description:         EXAT       EDESCRIPTION:	shall b	e the sai	me as detail	ed above for a	8" deck with $\emptyset A = 90^\circ$ ,			<b>_</b>			
<ul> <li>All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.</li> <li>All construction joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2'-6" Min. Iap length for Bars 4R3.</li> <li>The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.</li> <li>Description:</li> </ul>	prohibi	it placeme	ent, Bars 7P					Concrete			
A At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2-6" Min. Jap length for Bar 6R1, a 2'-0" Min. Jap length for Bars 5R2 and a 1'-3" Min. Jap length for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details. LAST E DESCRIPTION: LAST E DESCRIPTION:	3. All rei	inforcing	steel at the	e open joints sl	nall have a 2" minimum cover			5			ESTIMAT
to side, see Structures Plans, Superstructure Sheets for details. Typical 10'-0" Set Approach Slab wi Bar 7P4 ***	<ol> <li>At Cor spliced Bar 6R length</li> <li>The sl</li> </ol>	nstructior 1. Where 21, a 2'-0' for Bars kew angle	n Joints Bars bars are sp " Min. lap lei 4R3. e for Bars 7	oliced provide a ngth for Bars 5 'P3 may vary fi	a 2'-6" Min. lap length for 5R2 and a 1'-3" Min. lap rom joint to joint and side	to place Railing Perp				Lengt	ITE
LAST EVISION EFDOT DESIGN STANDARDS	to side	e, see Str	ructures Plai	ns, Superstruct	ure Sheets for details.				5,-6	Typical .	10'-0" Section w
LAST EVISION EFFOT DESIGN STANDARDS										Approacl	h Slab with Gua
LAST EVISION STANDARDS											
EVISION ISI I I I I I I I I I I I I I I I I I									Bar 7P4 ***		
7/01/05 FY 2012/2013 TRAFFIC RAILING - (	REVISION	11510	IPTION:			8°316		DARDS	TRA	AFFIC RAILIN	G - (COR

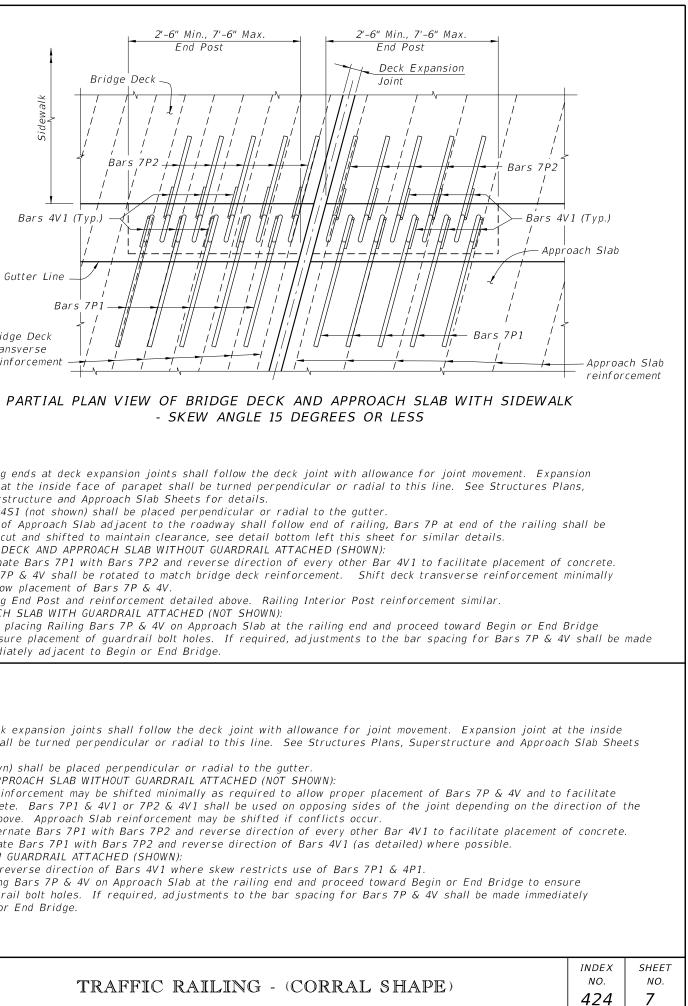










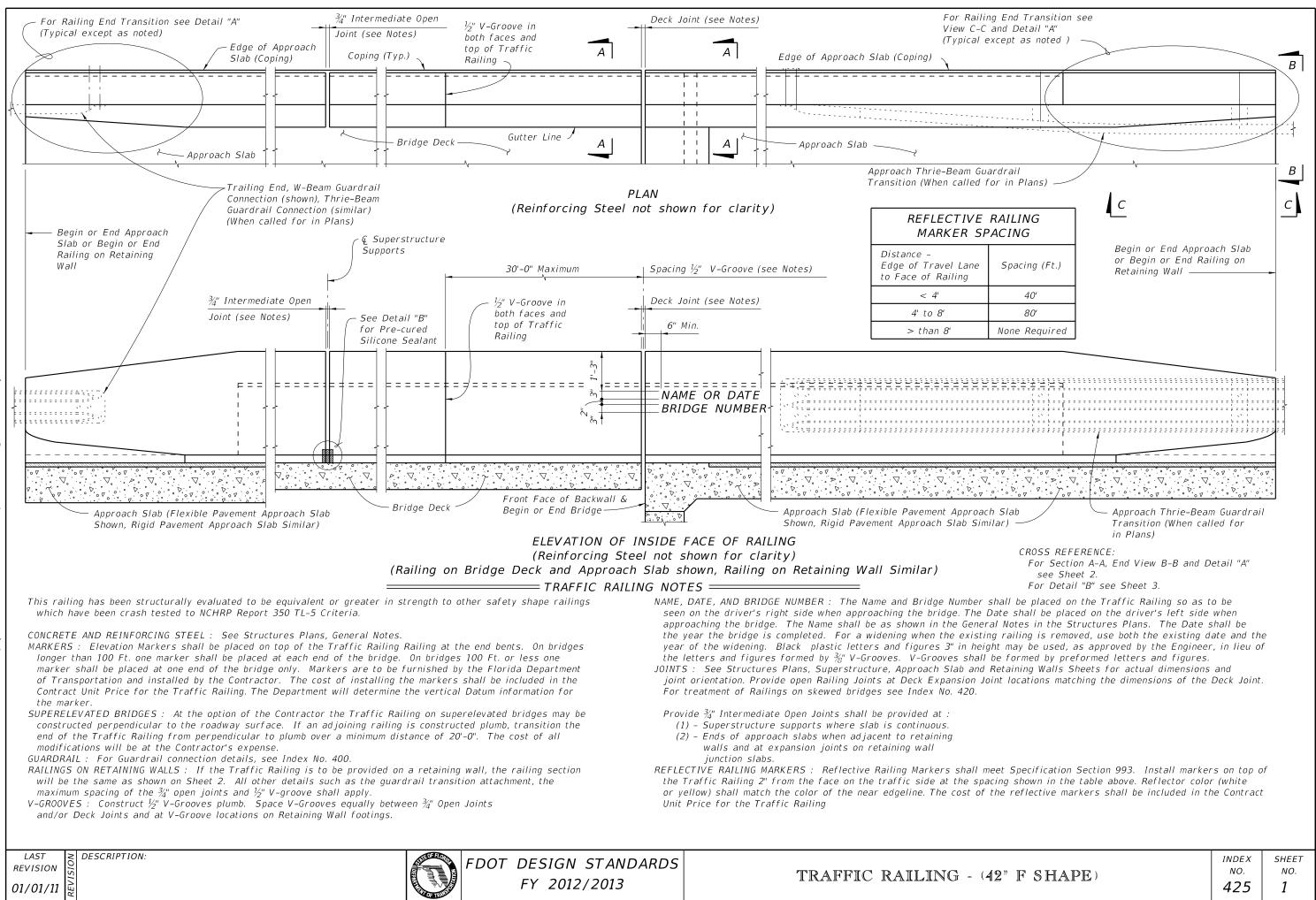


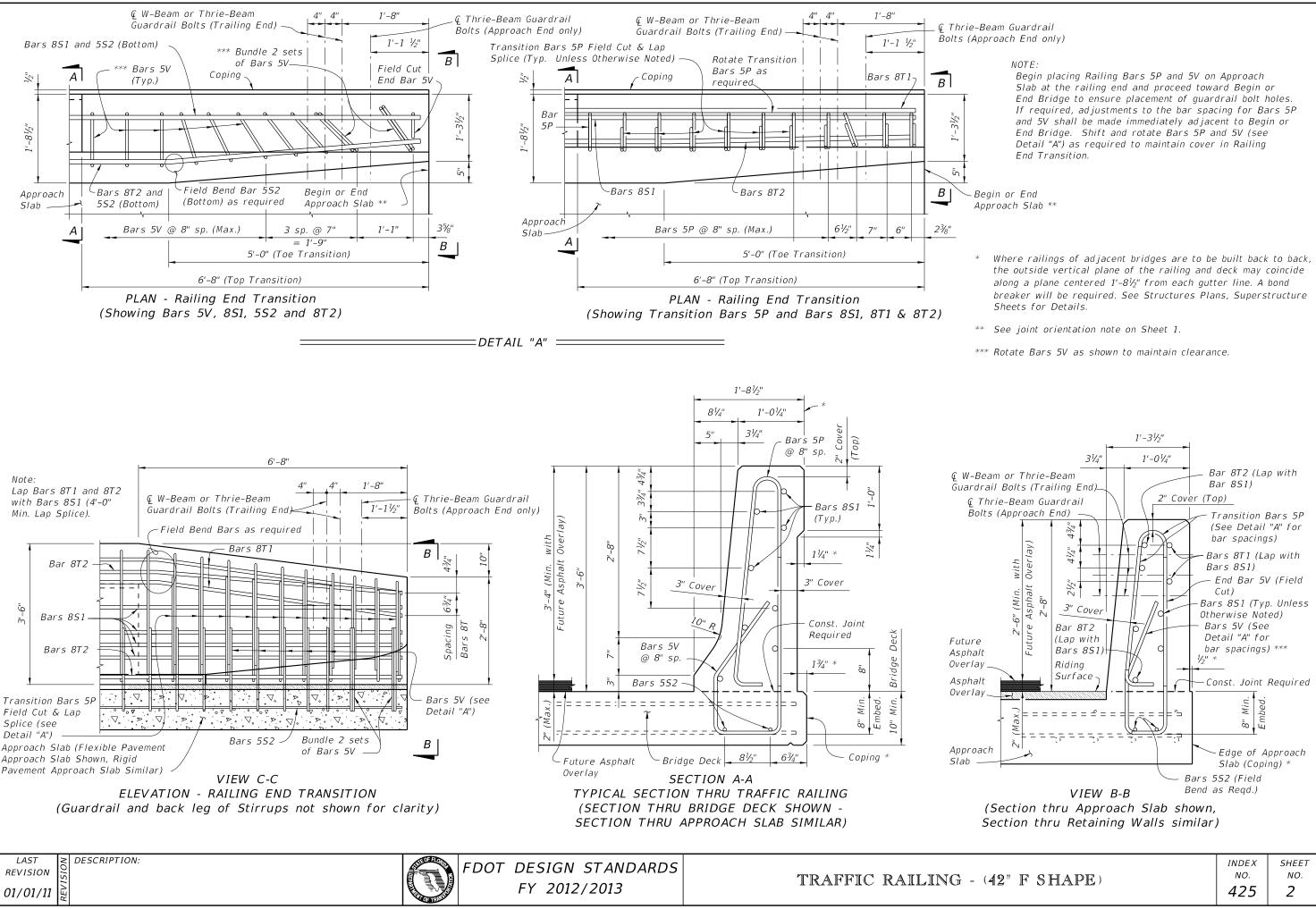
NOTES:

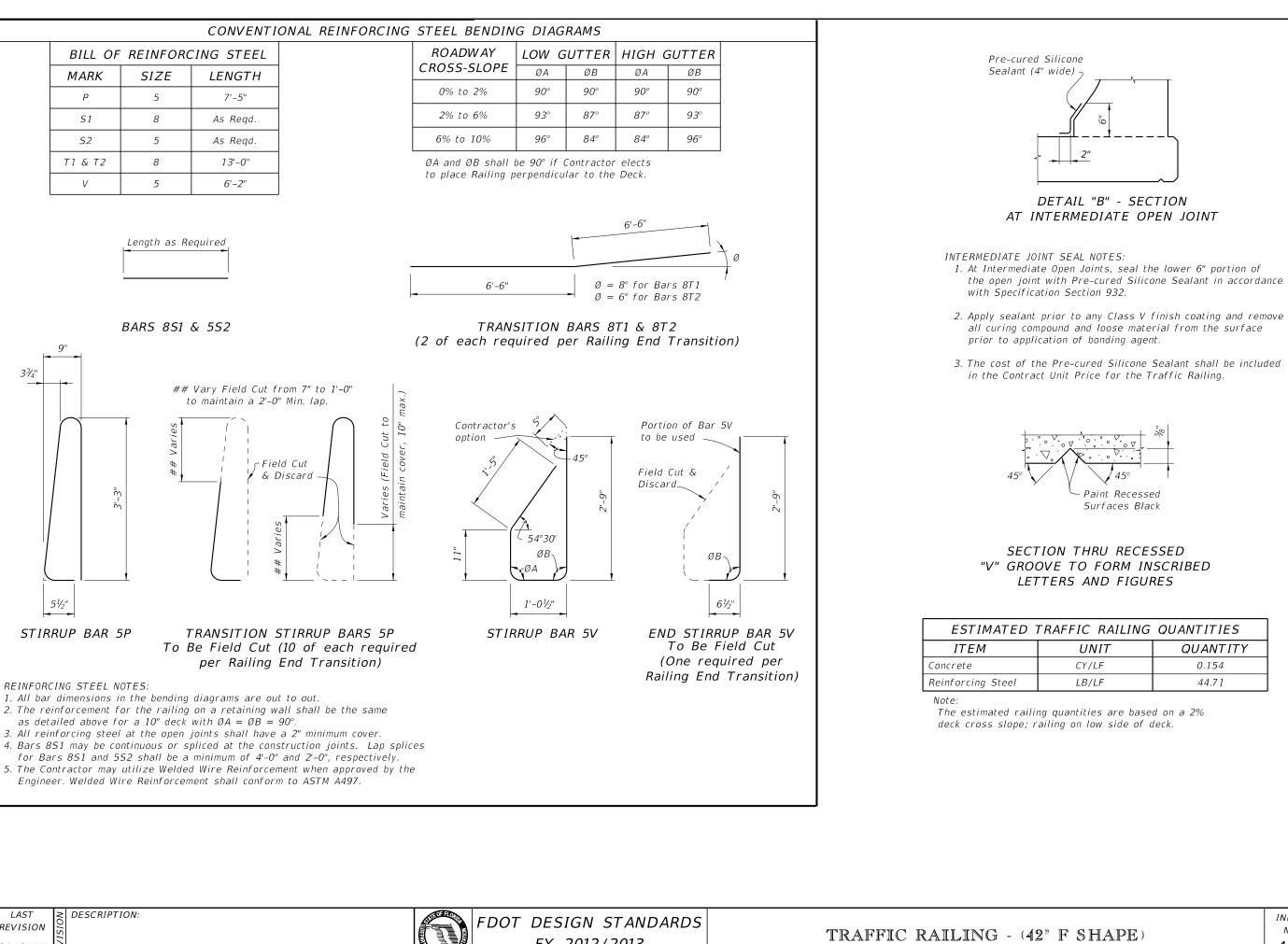
- Superstructure and Approach Slab Sheets for details.

- for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (NOT SHOWN):

- adjacent to Begin or End Bridge.







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# DETAIL "B" - SECTION

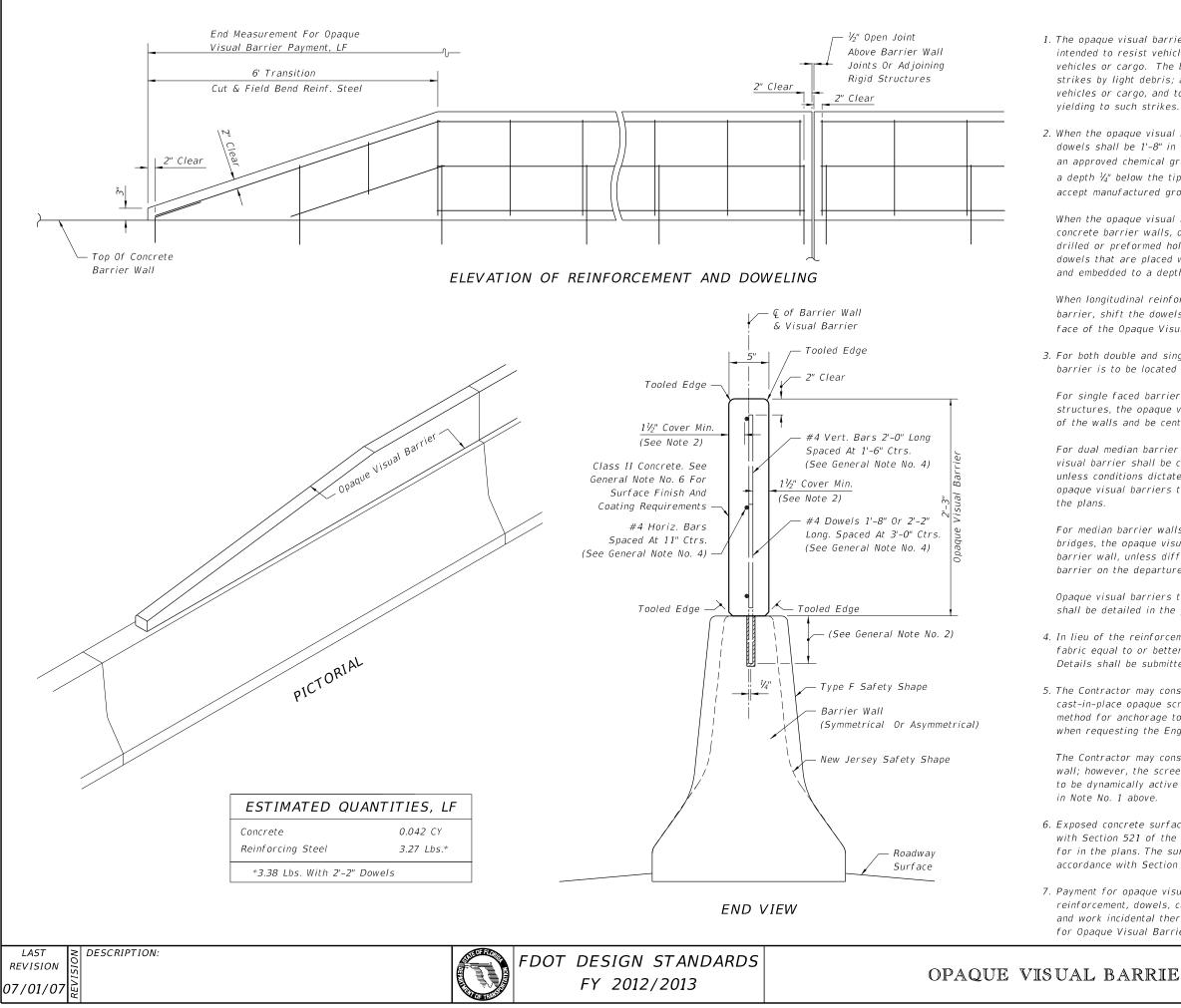
the open joint with Pre-cured Silicone Sealant in accordance

all curing compound and loose material from the surface

3. The cost of the Pre-cured Silicone Sealant shall be included

RAFFIC RAILING	QUANTITIES
UNIT	QUANTITY
CY/LF	0.154
LB/LF	44.71

2" F SHAPE)	INDEX NO.	SHEET NO.
z I OIIAEL	425	3



## GENERAL NOTES

1. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when

2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved chemical grout. Embedment holes shall be  $\frac{5}{6}$ " diameter, drilled to a depth  $\frac{1}{4}$ " below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opague visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the  $1\frac{1}{2}$ " Cover Minimum to the face of the Opaque Visual Barrier.

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

4. In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer. Details shall be submitted with requests for substitution.

5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineer's approval.

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations

6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finish Coating in accordance with Section 400 only when called for in the plans.

7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.

	INDEX	SHEET
ARRIER	NO.	NO.
	461	1

== TRAFFIC RAILING NOTES ======

This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6". Field drilled holes for Post connections shall be  $\frac{3}{4}$ " by 2<sup>1</sup>/<sub>2</sub>" slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive-Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM 449. All Nuts shall be single self-locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

COATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for % of anchor bolts; 55,000 lbs. for the 14 anchor bolts with 13 embedment; and 30,500 lbs. for the  $1\frac{1}{4}$ " Ø anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

POST SPACING: Posts shall be located along the length of the bridge at typical 6'-3'' or  $3'-1\frac{1}{2}''$  spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten quardrail bolts in  $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

NEOPRENE PADS: Neoprene pads must be plain pads with a durometer hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required.

- ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The Department will determine the vertical Datum information for the marker.
- REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall conform to Section 993 of the Specifications. Install markers at the top of the guardrail posts at the spacings shown in the table below. Reflector color (white or yellow) shall conform to the color of the near edgeline.
- PEDESTRIAN SAFETY PIPE RAIL: Pedestrian Safety Pipe Rail is required when called for in the Plans. See Index No. 400 for details.
- BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travel lane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.
- PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. The Pedestrian Safety Pipe Rail, Transition Blocks and Curbs, Bridge Name Plate, Reflective Railing Markers and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

REFLECTIVE F MARKER SPA	
Distance – Edge of Travel Lane to Face of Railing	
< 4'	
4' to 8'	
> than 8'	

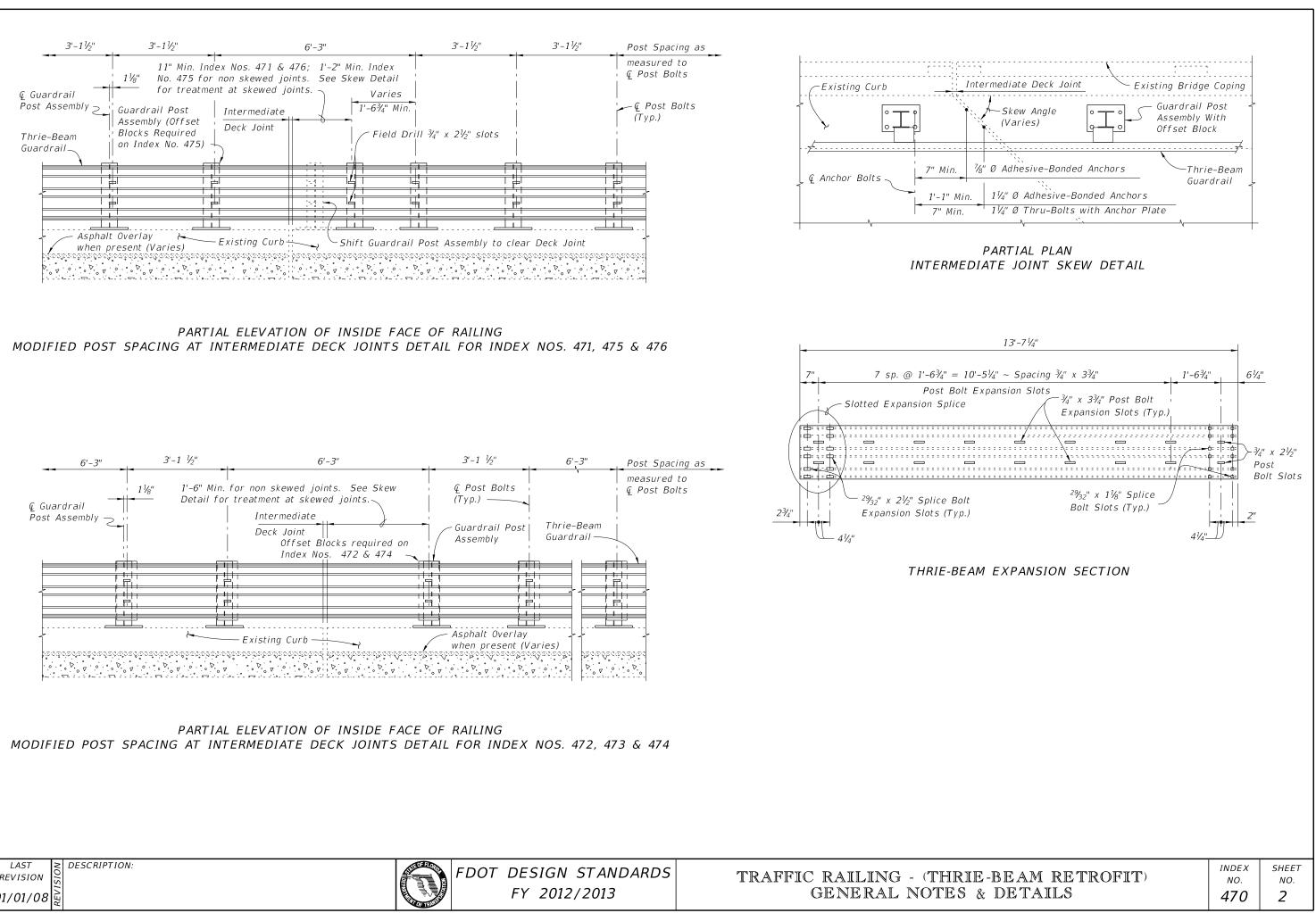
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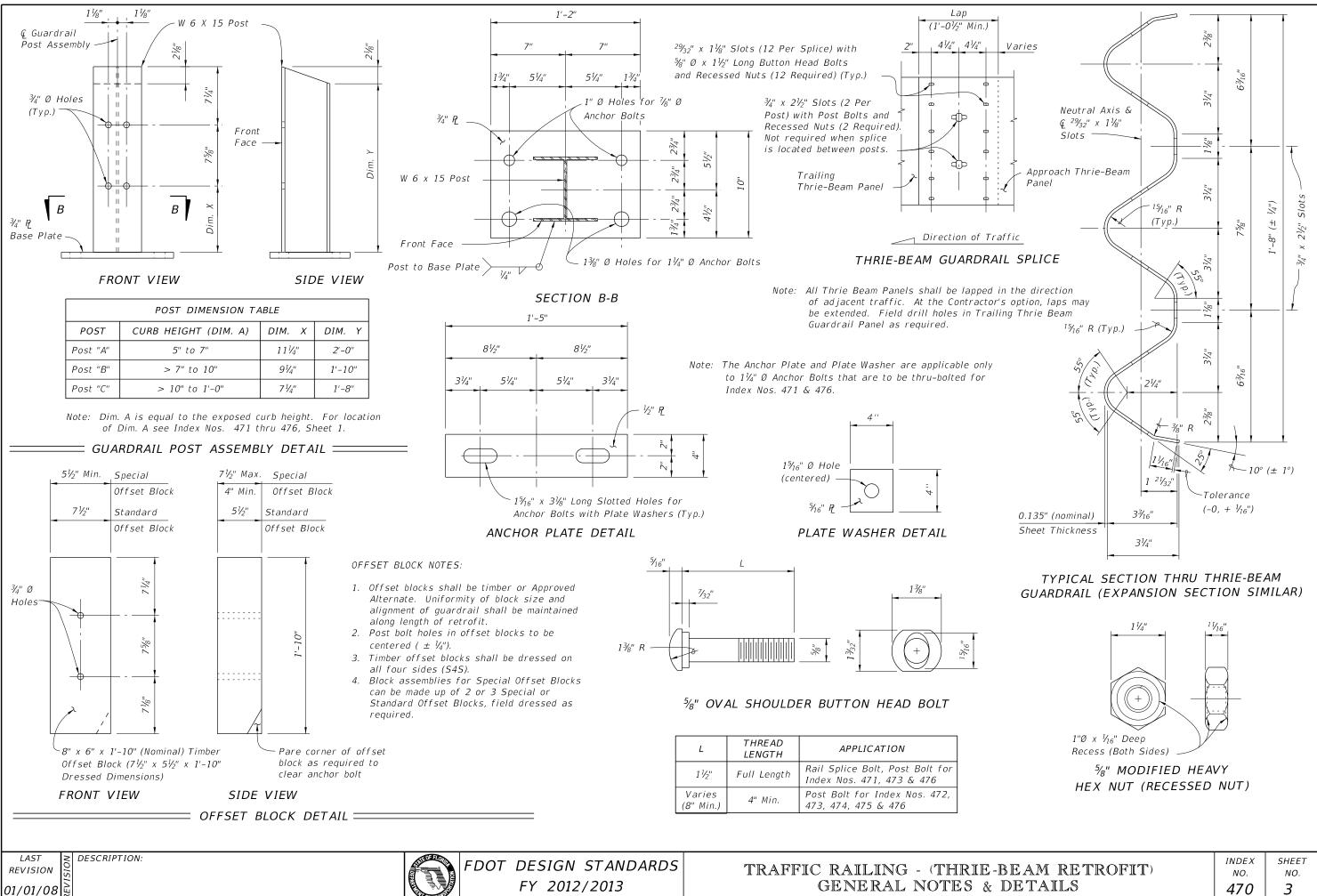
NILING CING
Spacing (Ft.)
40'
80'
None Required

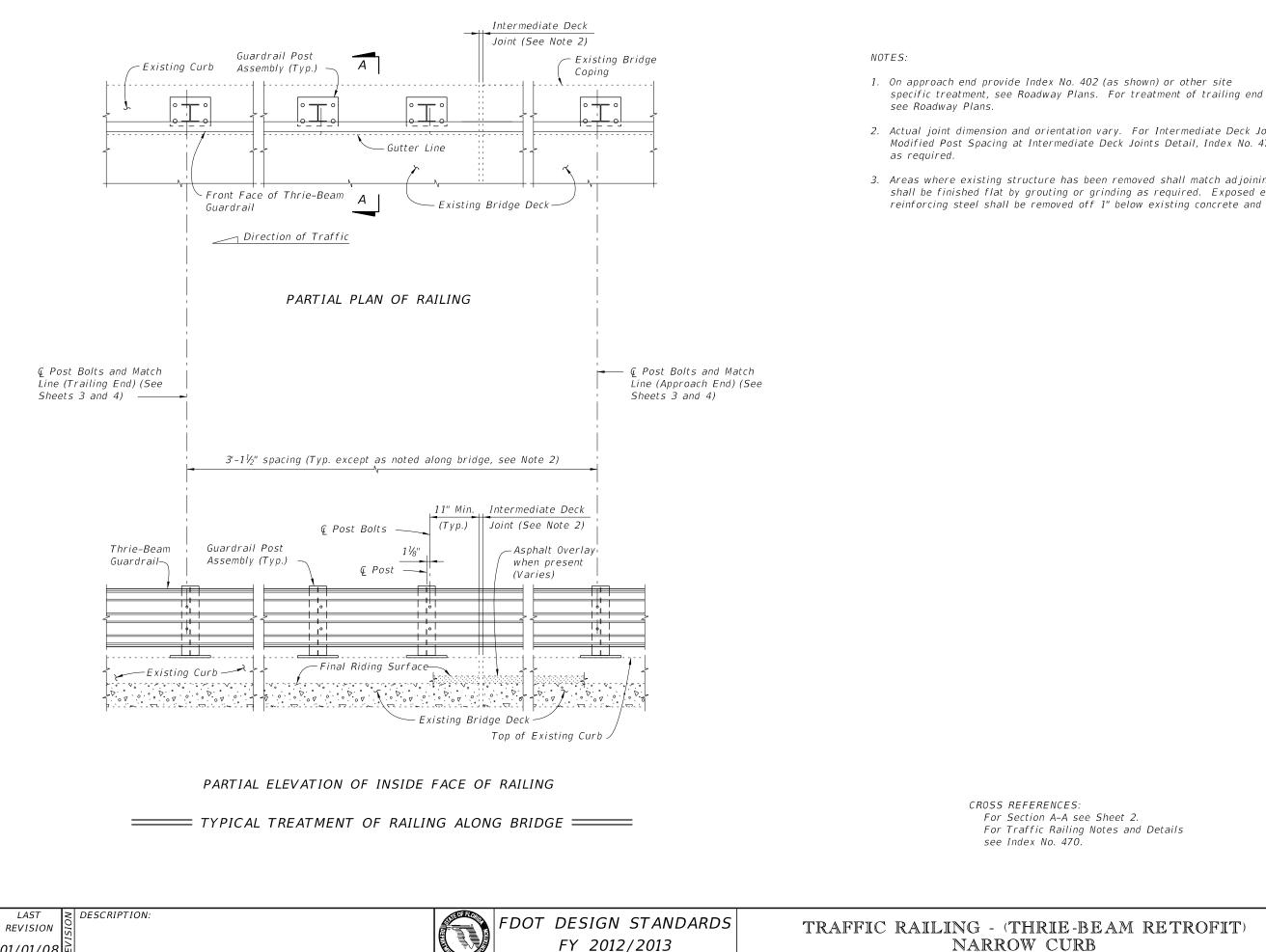
EAM RETROFIT	INDEX NO.	SHEET NO.
DETAILS	470	1



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COLUMN TRANSPORT	





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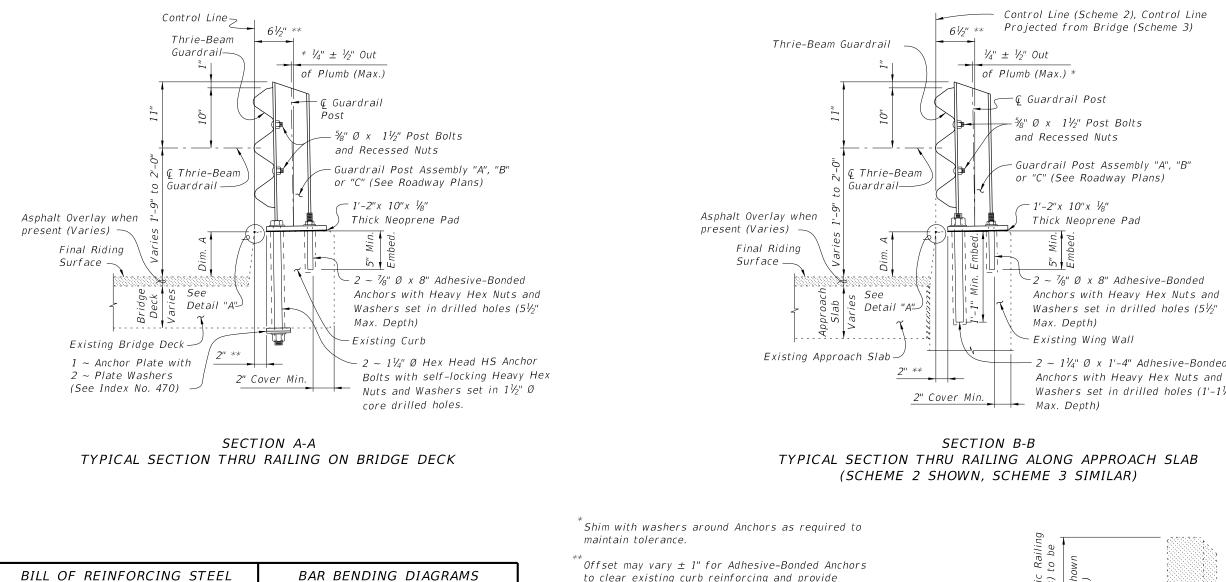
NARROW CUR

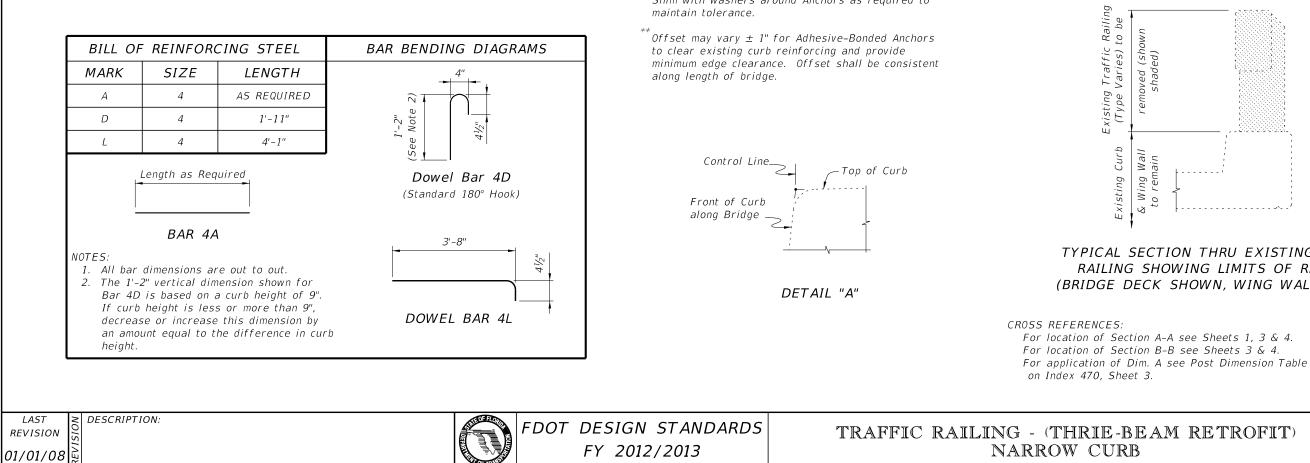
2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2,

3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be removed off 1" below existing concrete and grouted over.

For Traffic Railing Notes and Details

BEAM RETROFIT	INDEX NO.	SHEET NO.
RB	471	1



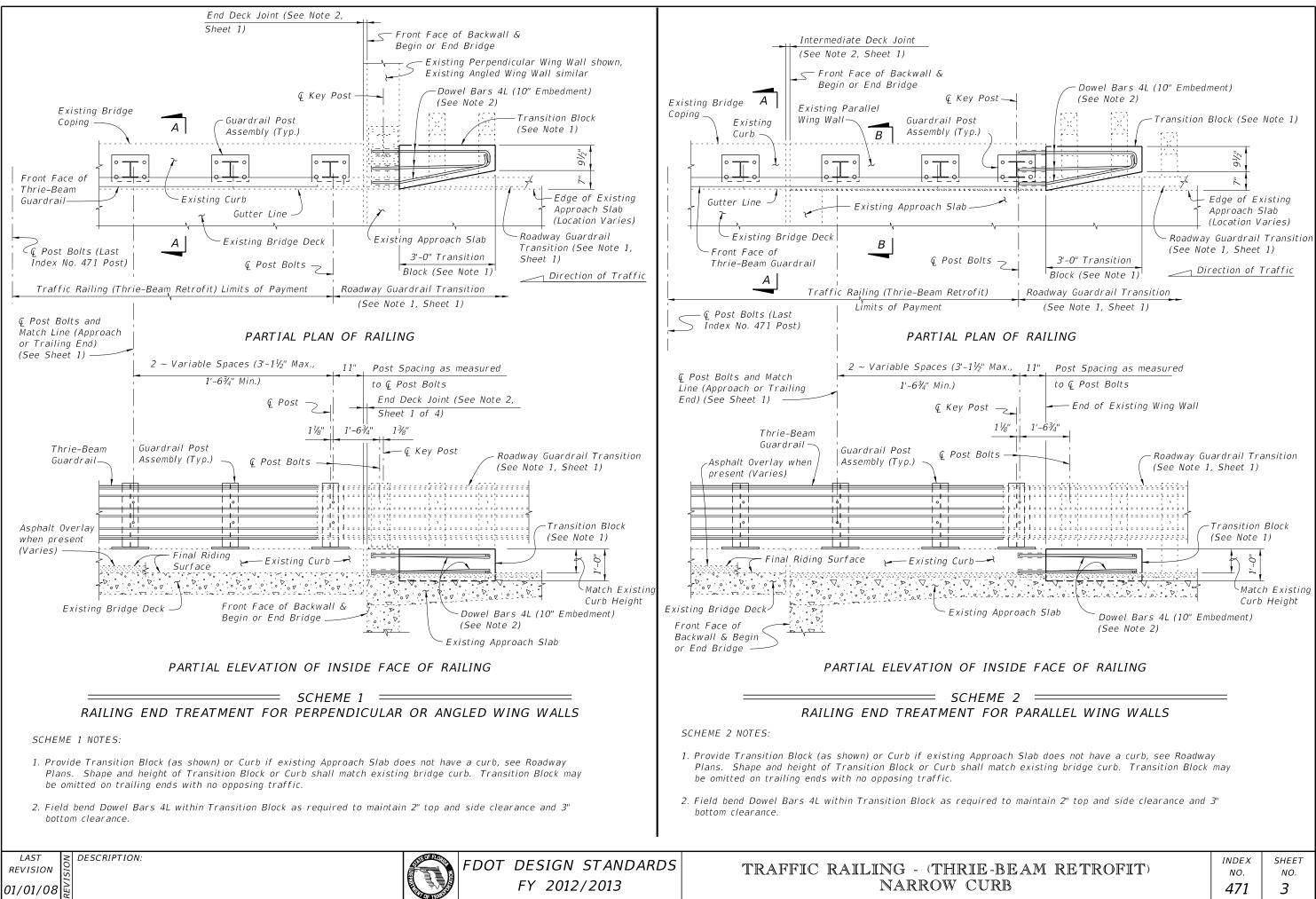


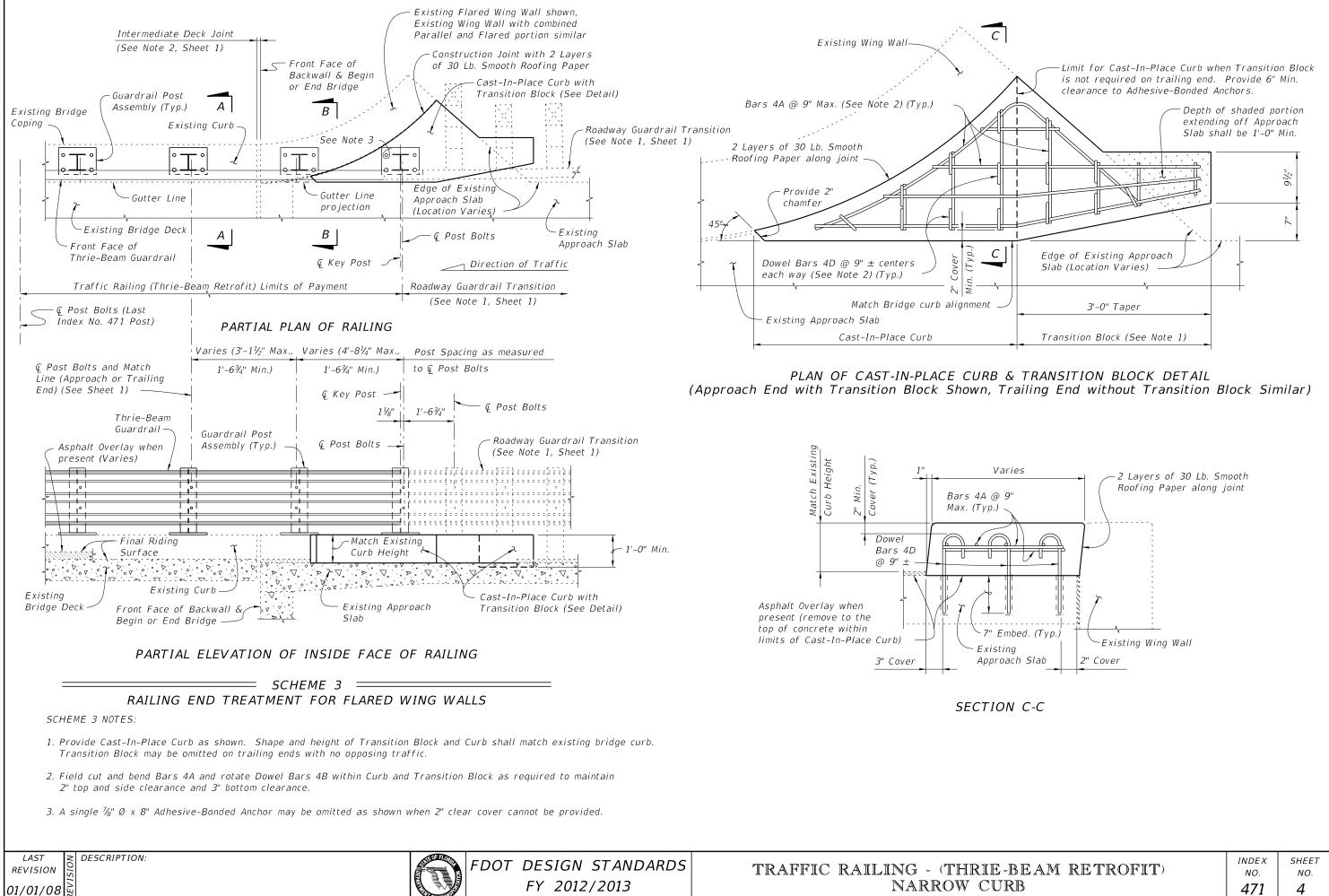
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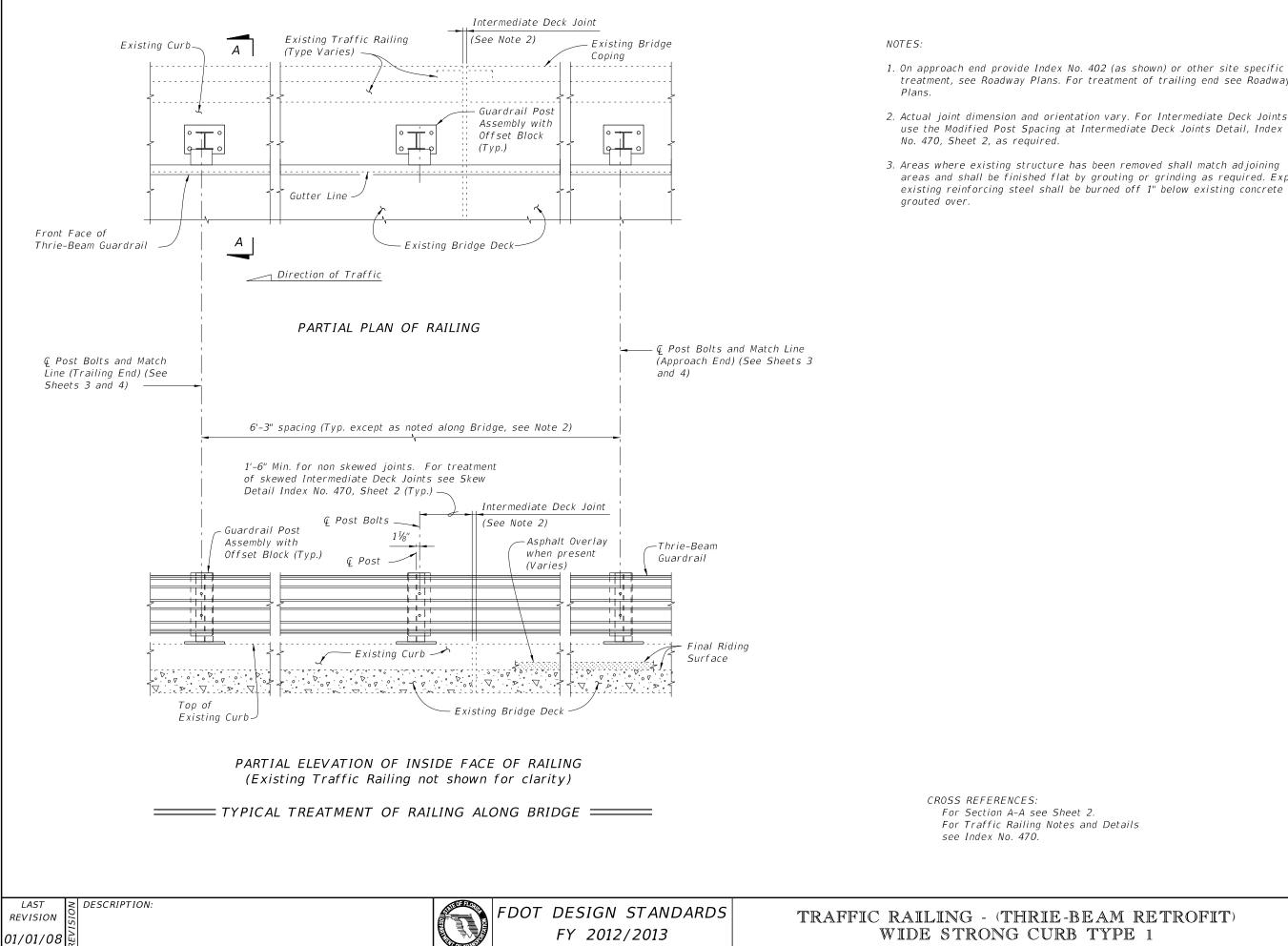
 $2 \sim 1^{1}/_{4}^{"} \varnothing \times 1^{'}-4^{"}$  Adhesive-Bonded Washers set in drilled holes  $(1'-1\frac{1}{2}'')$ 

# TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

BEAM RETROFIT)	INDEX NO.	SHEET NO.
RB	471	2





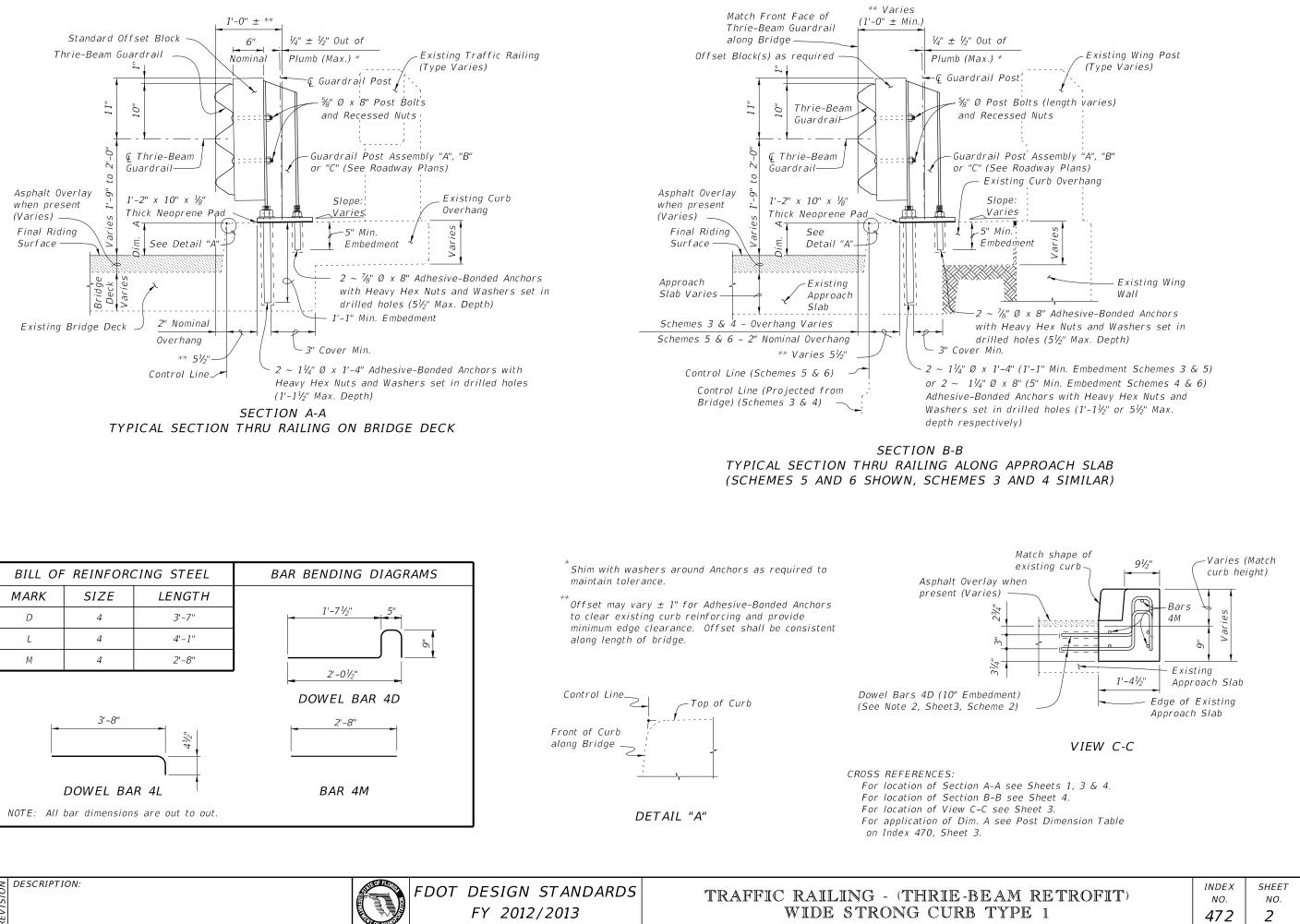


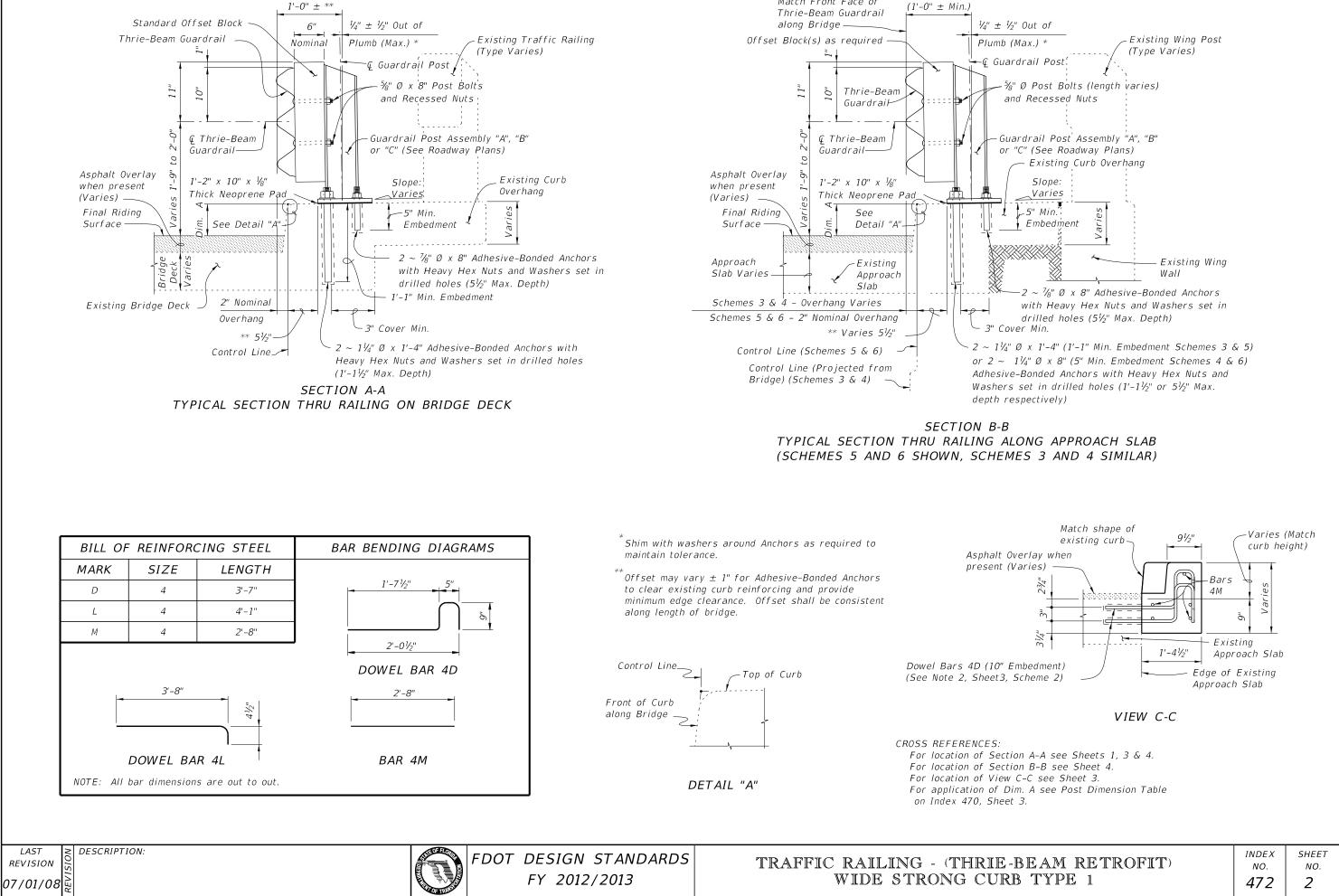
WIDE STRONG CURB TYPE 1

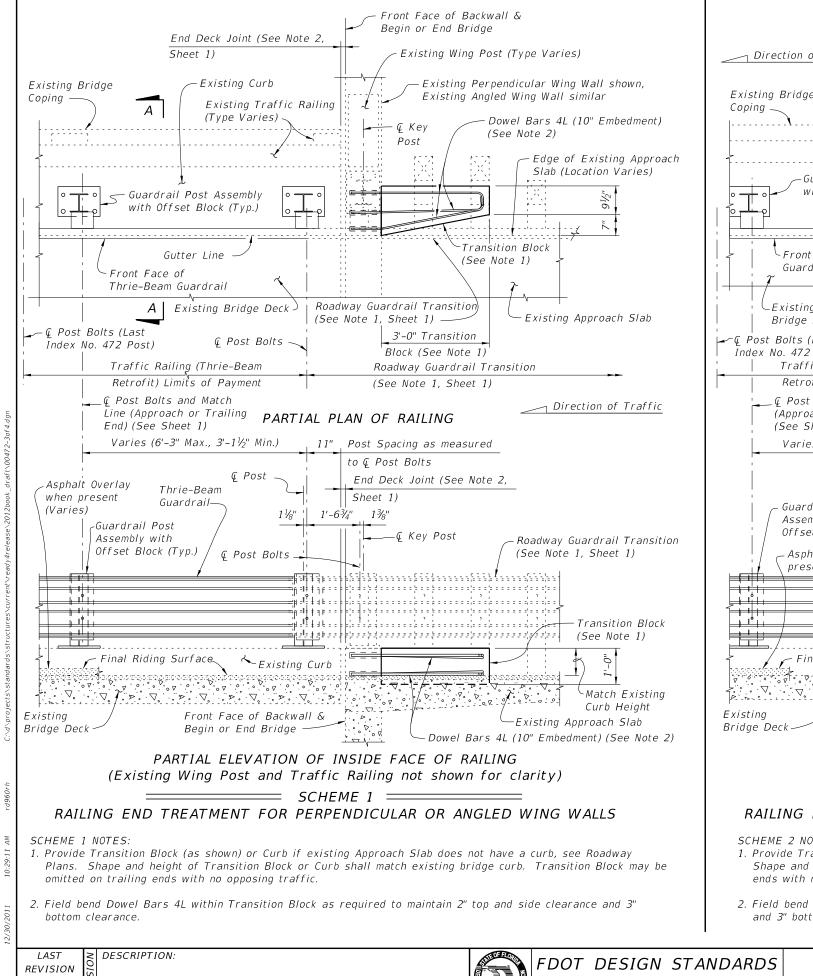
treatment, see Roadway Plans. For treatment of trailing end see Roadway

2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index

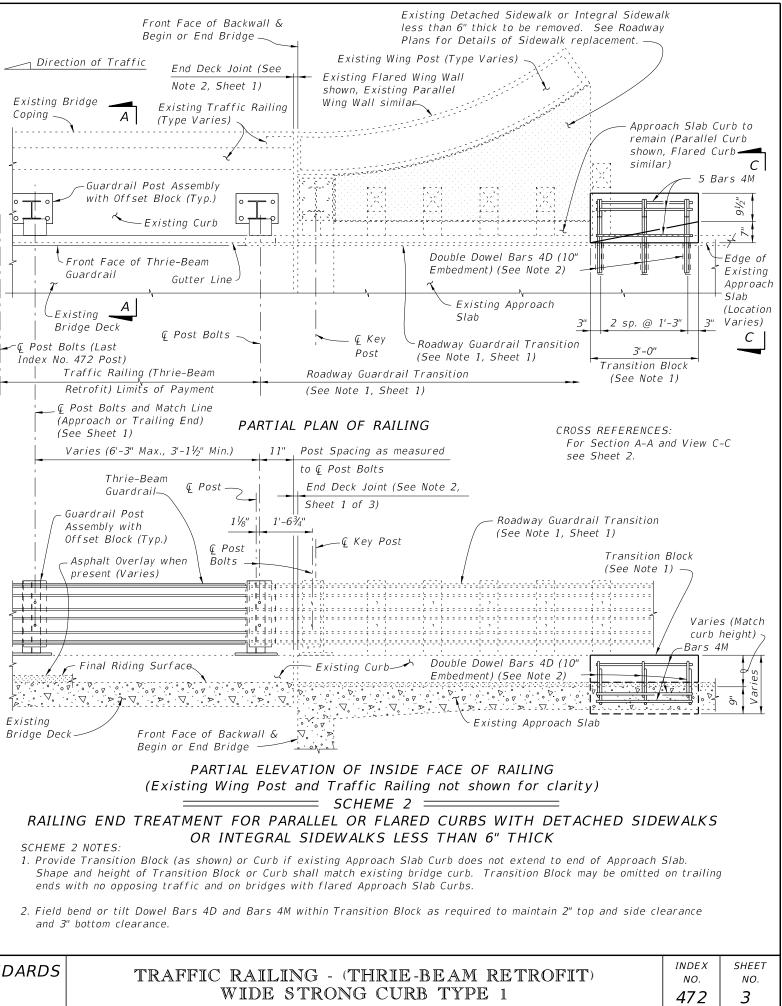
3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and



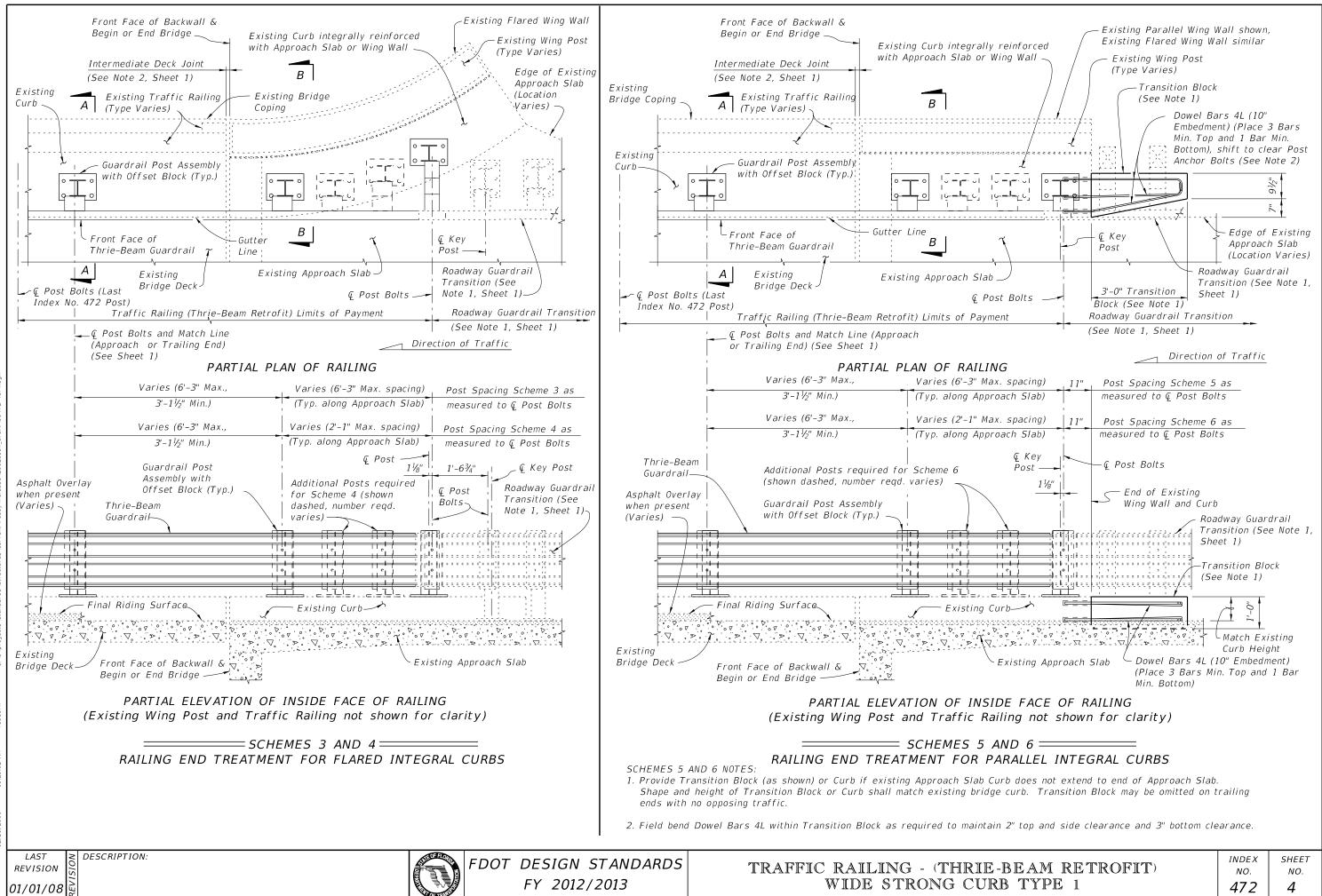




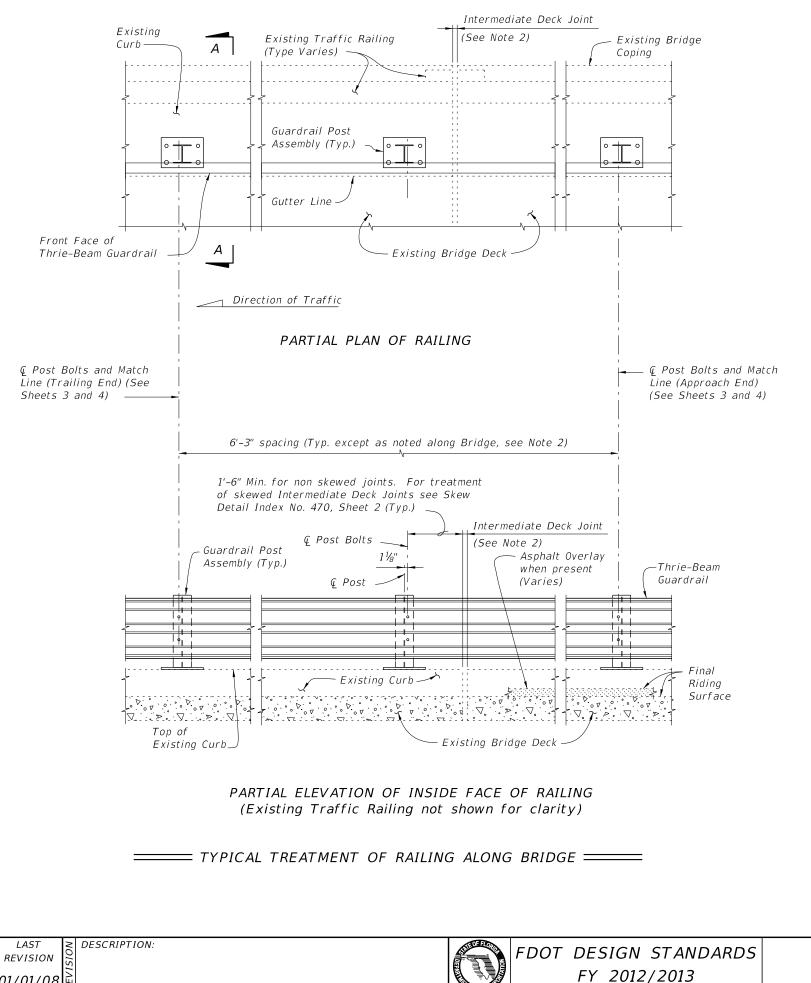
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## NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be be burned off 1" below existing concrete and grouted over.

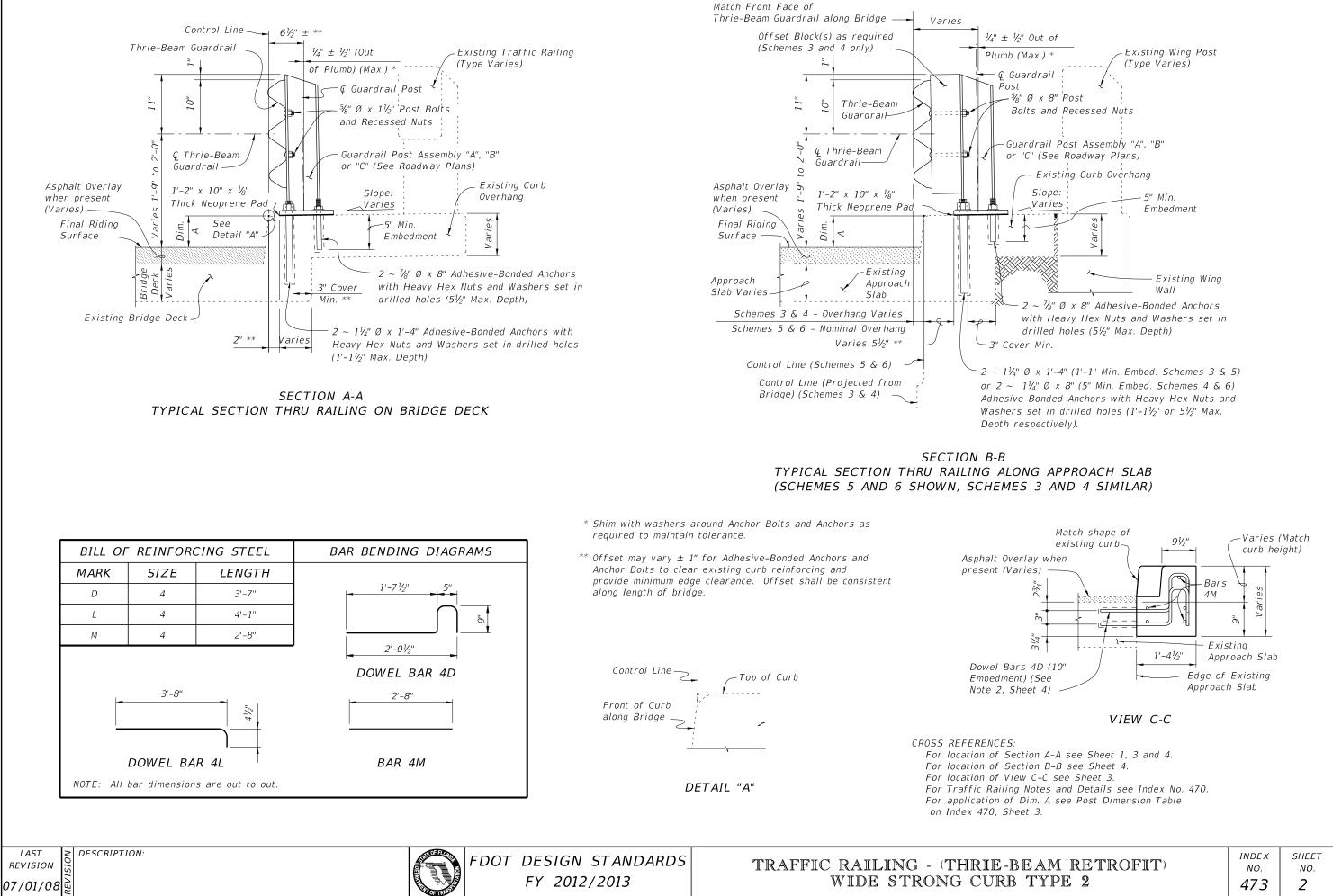
2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified

finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall

For Section A-A see Sheet 2. For Traffic Railing Notes and Details

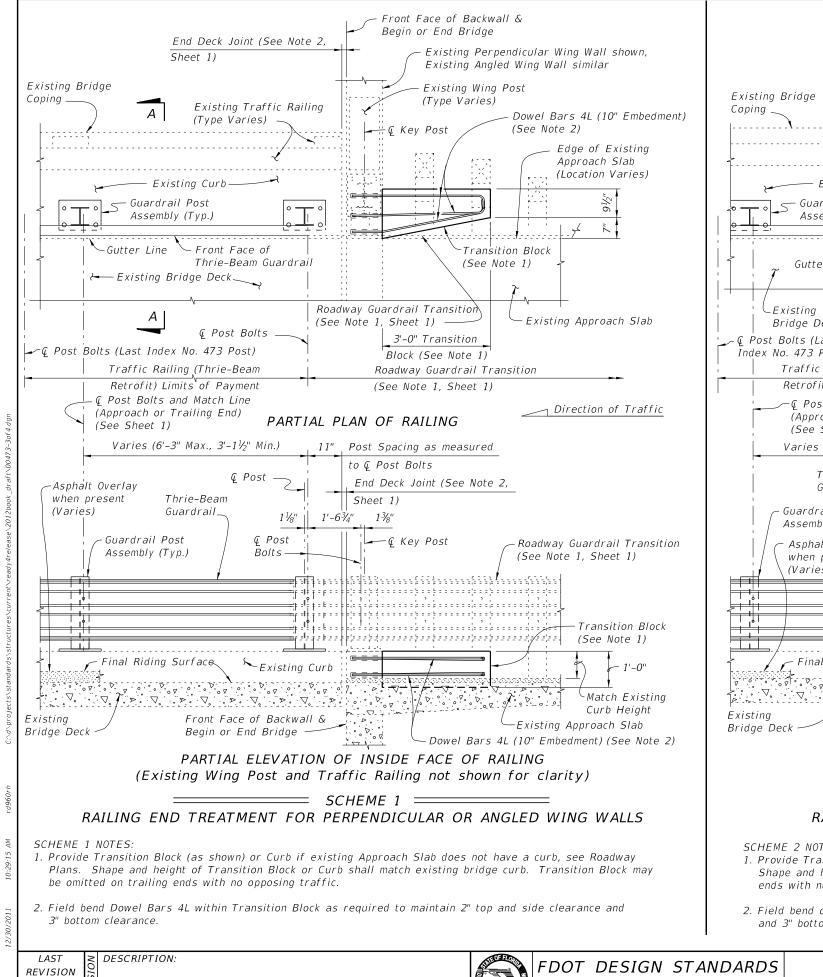
TRAFFIC RAILIN	NG - (THRIE-BEAM	RETROFIT)
WIDE ST	TRONG CURB TYPE	C 2

INDEX SHEET NO. NO. 473 1

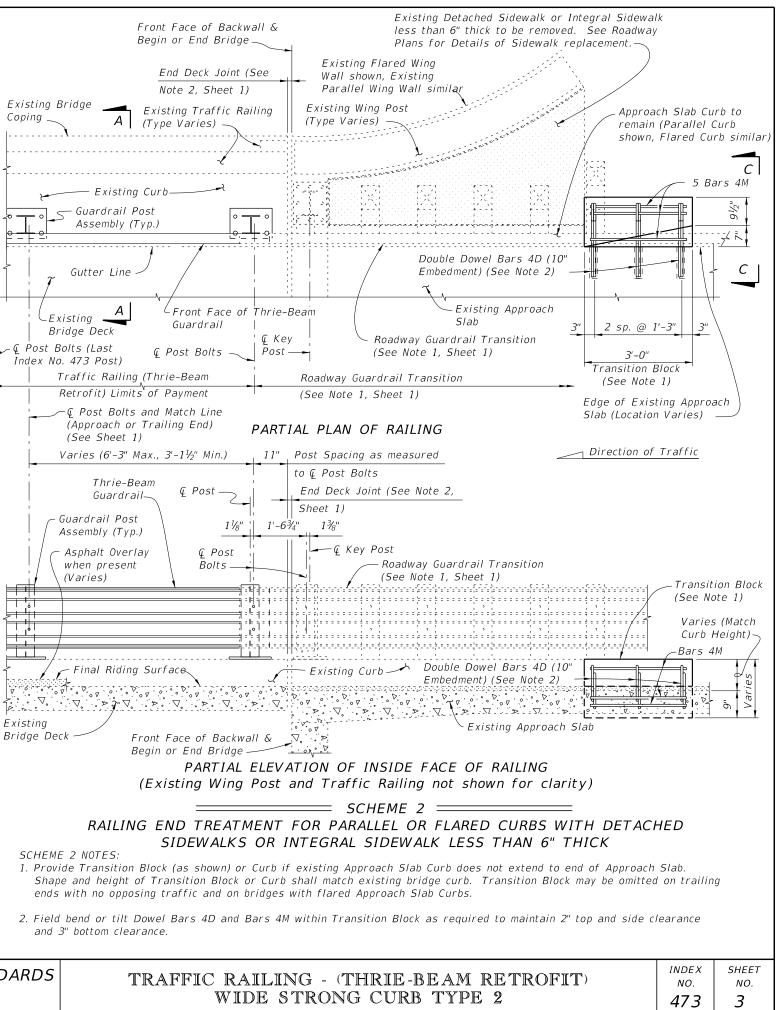




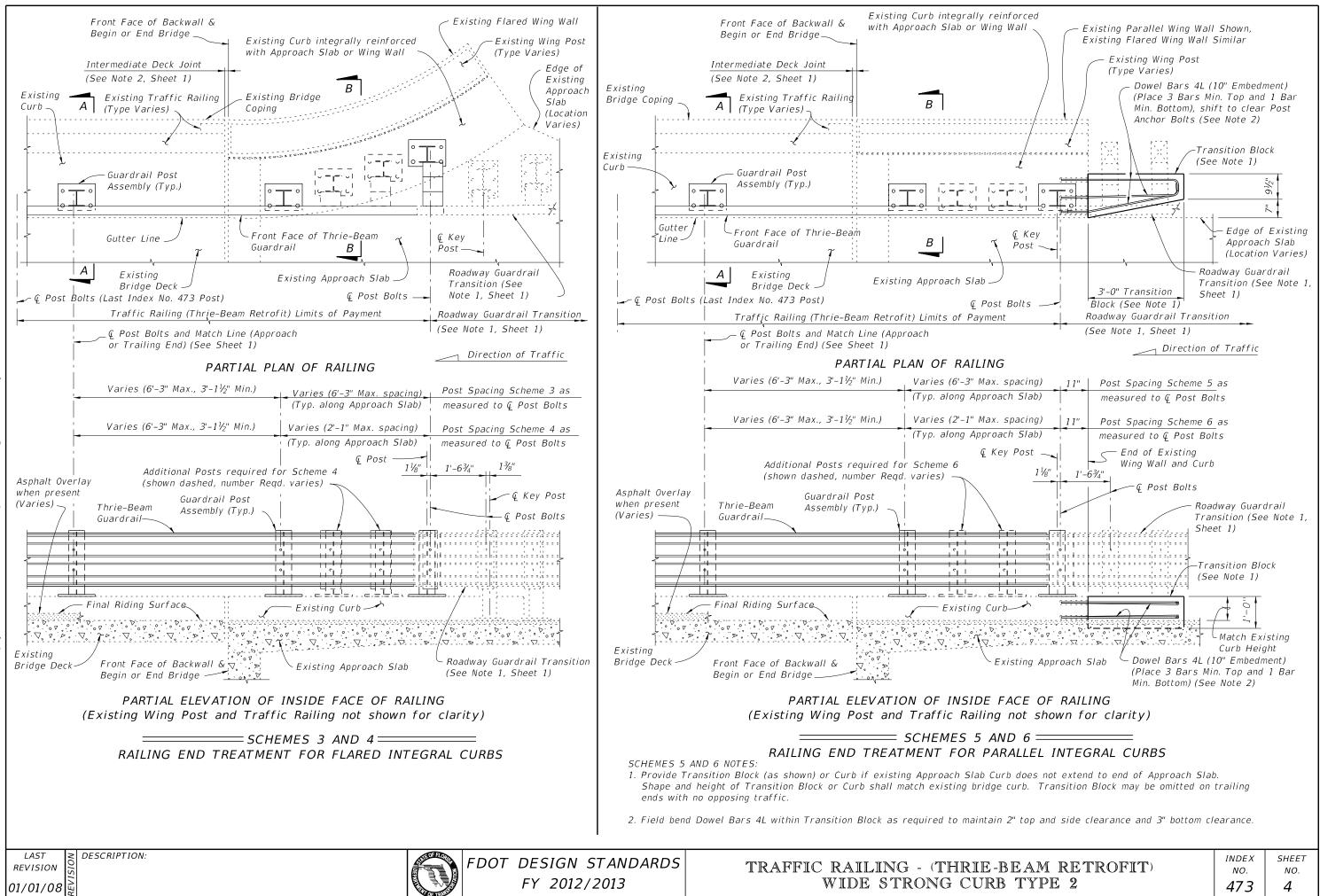




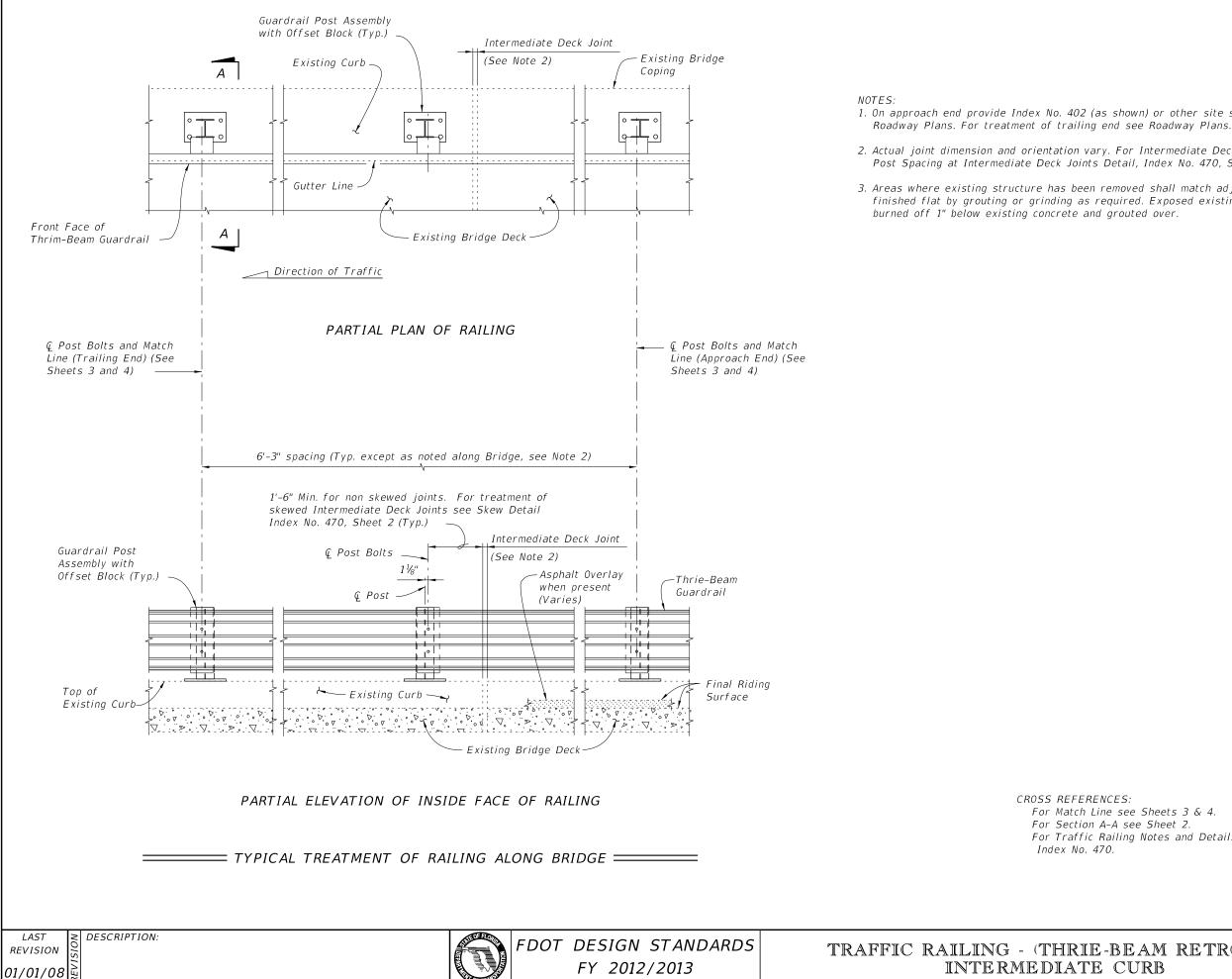
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INTERMEDIATE

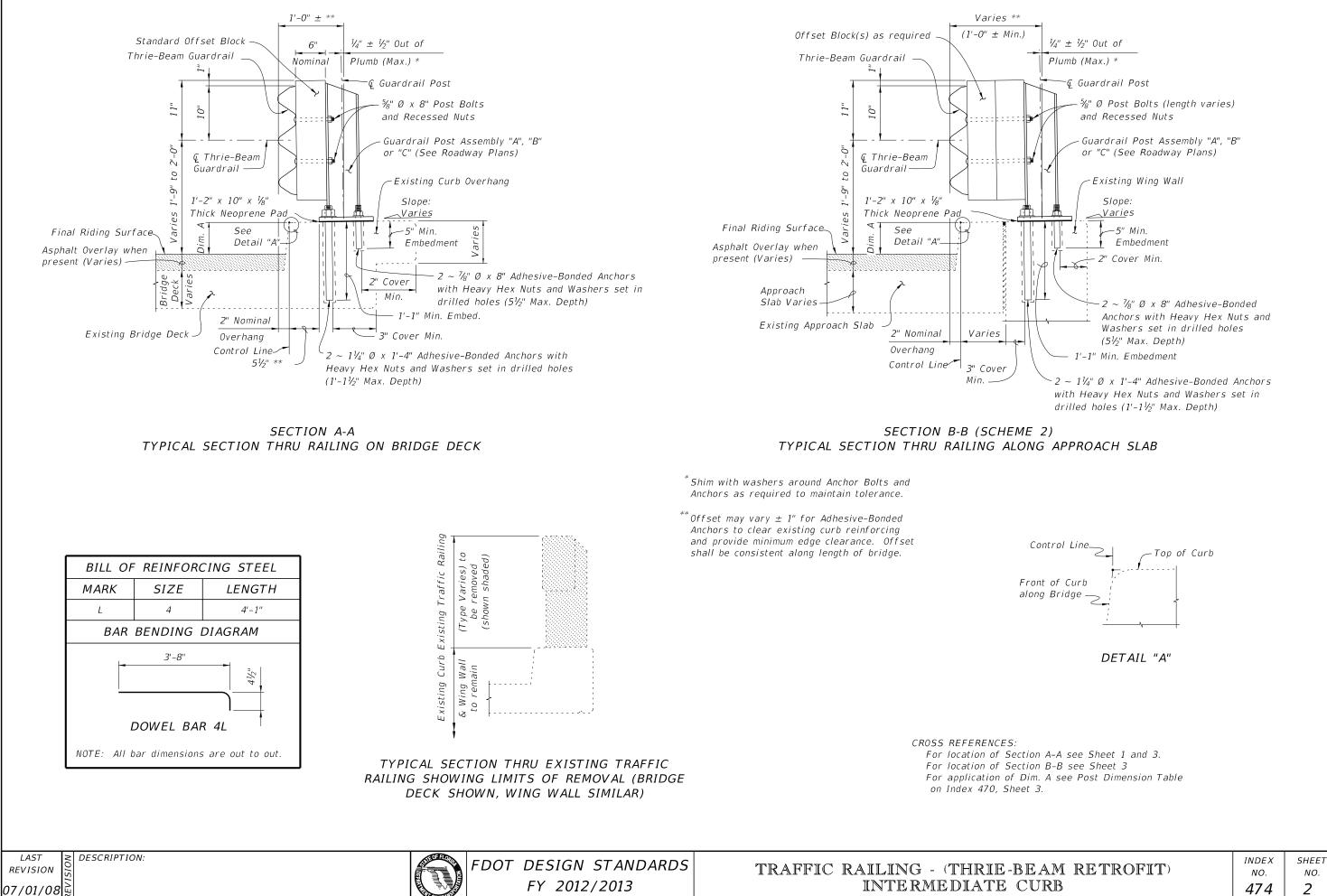
1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see

2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.

3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be

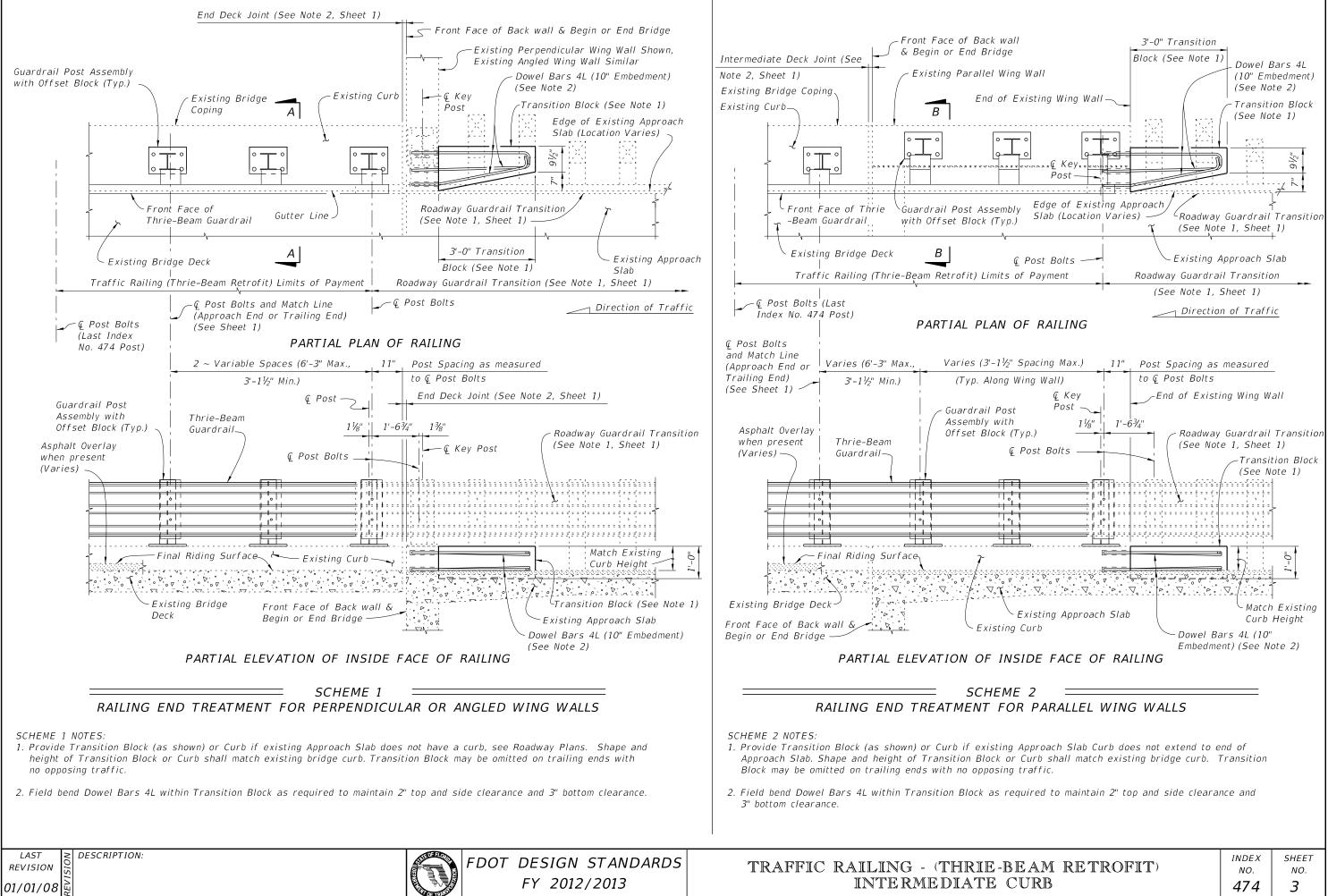
> For Match Line see Sheets 3 & 4. For Section A-A see Sheet 2. For Traffic Railing Notes and Details see

BEAM RETROFIT	INDEX NO.	SHEET NO.
CURB	474	1

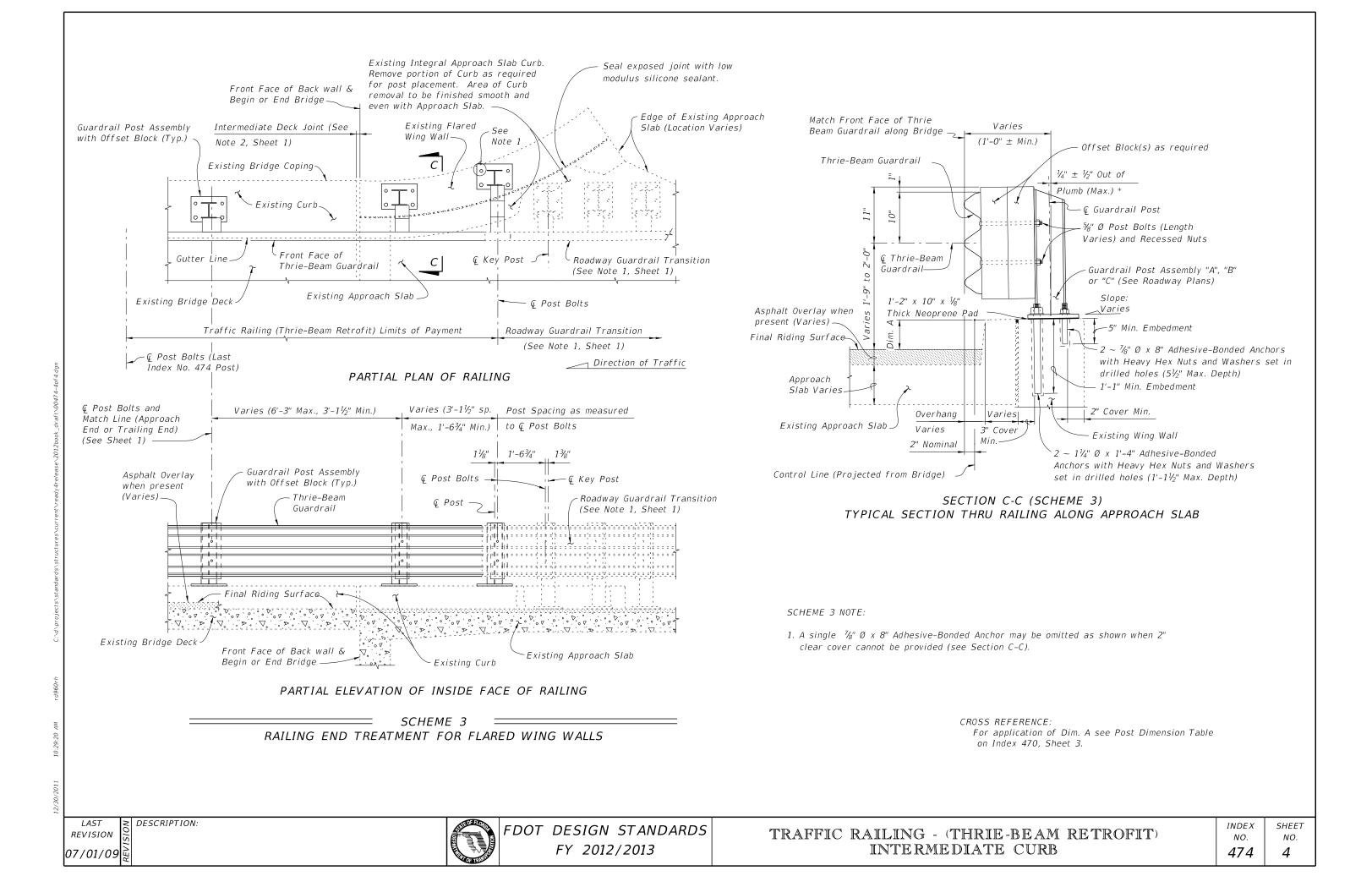


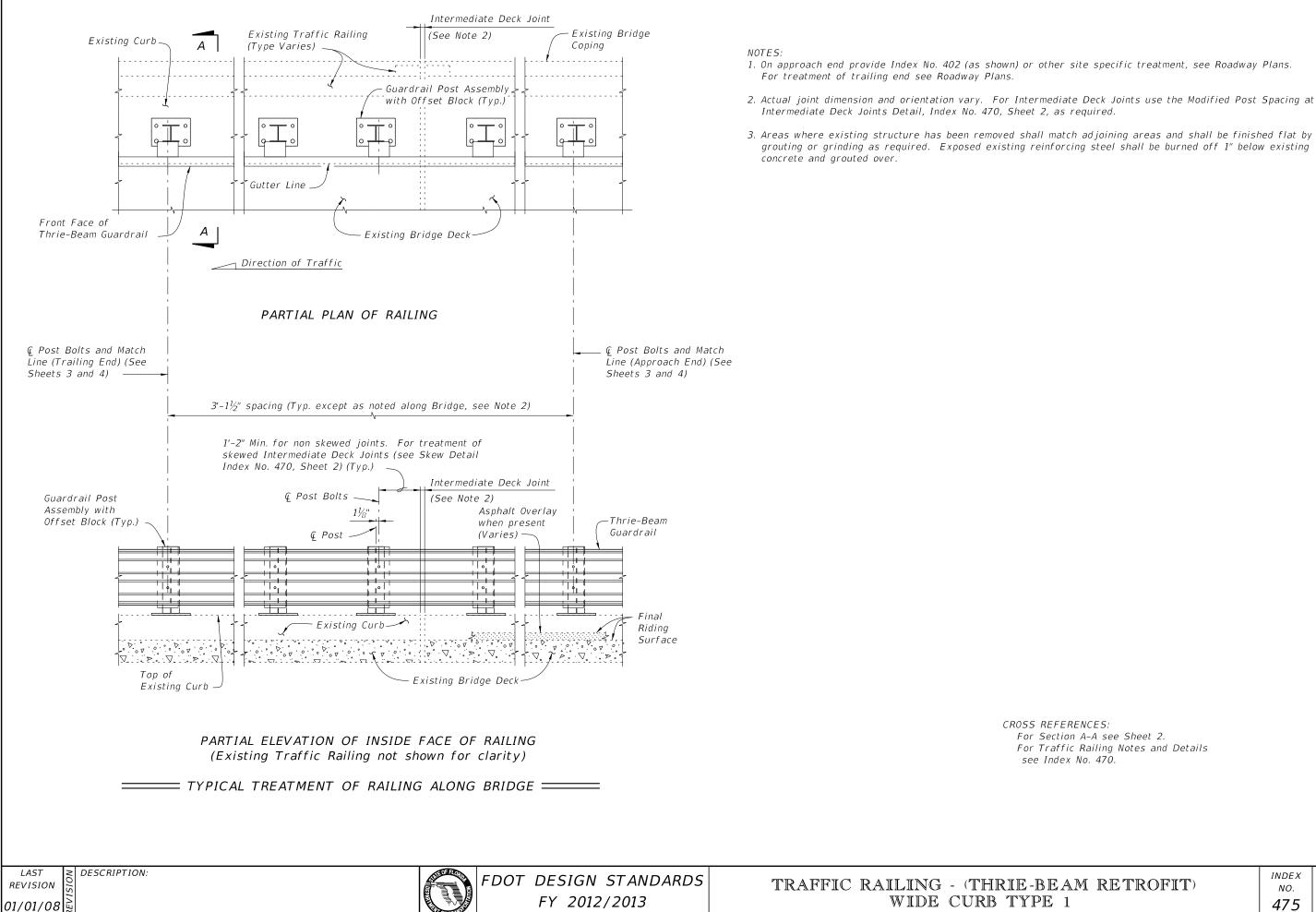
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INTERMEDIATE CURB



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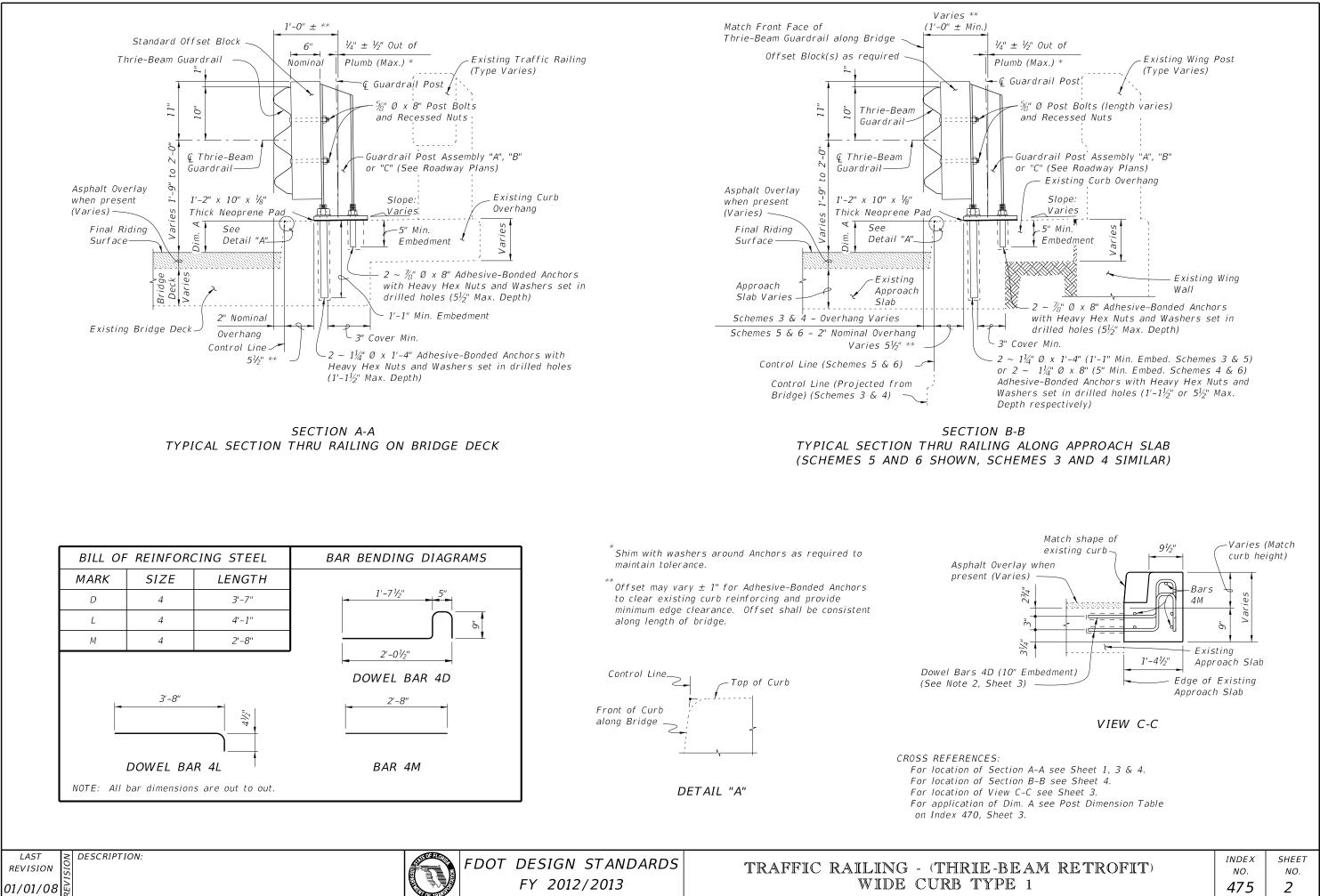
WIDE CURB TYP

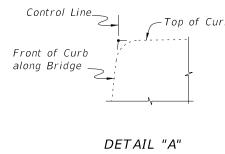
2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at

grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing

CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

BEAM PE 1	RETROFIT)	index no. <b>475</b>	sheet NO. <b>1</b>

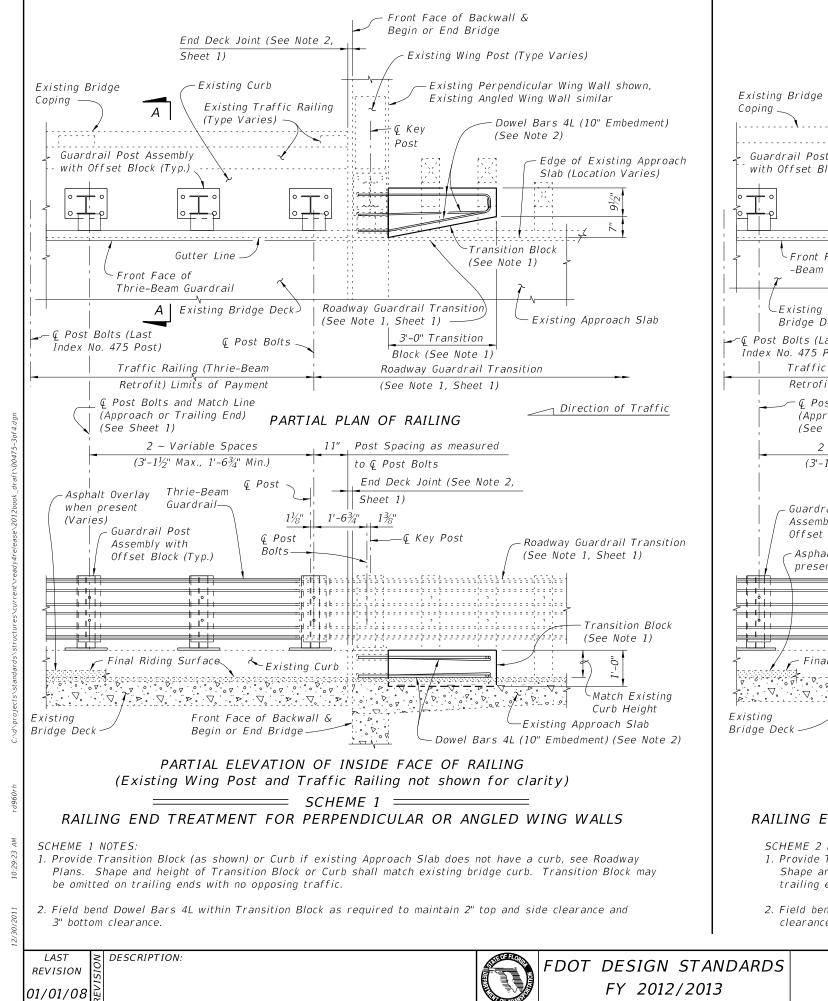


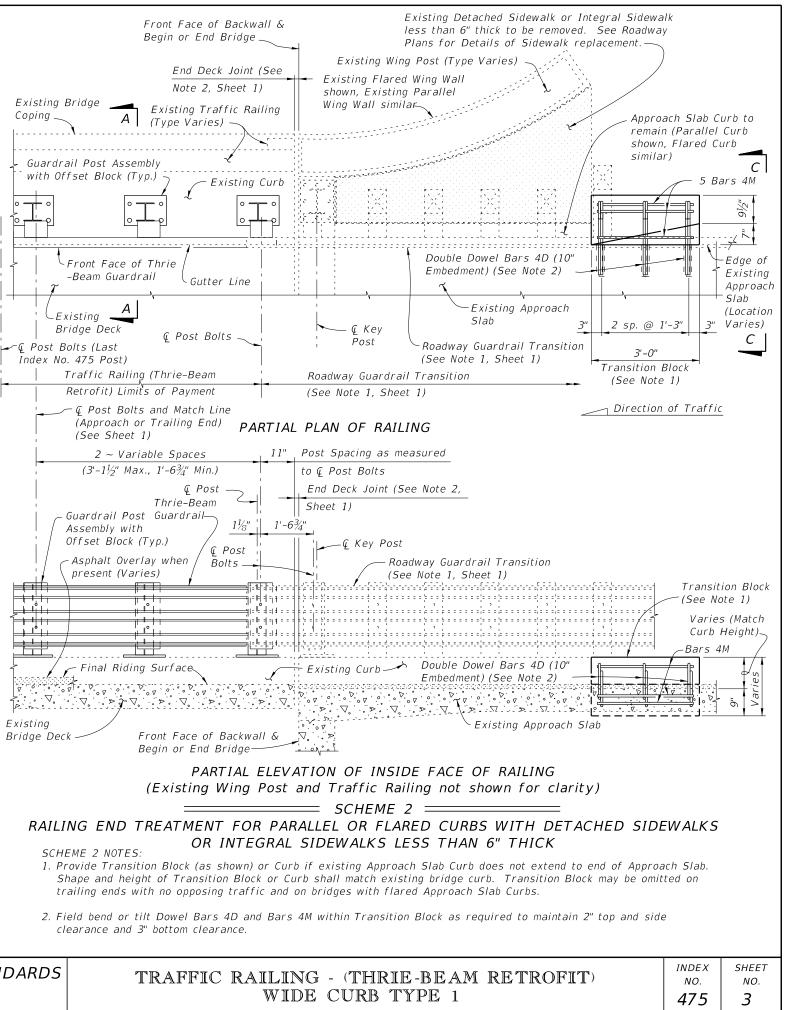


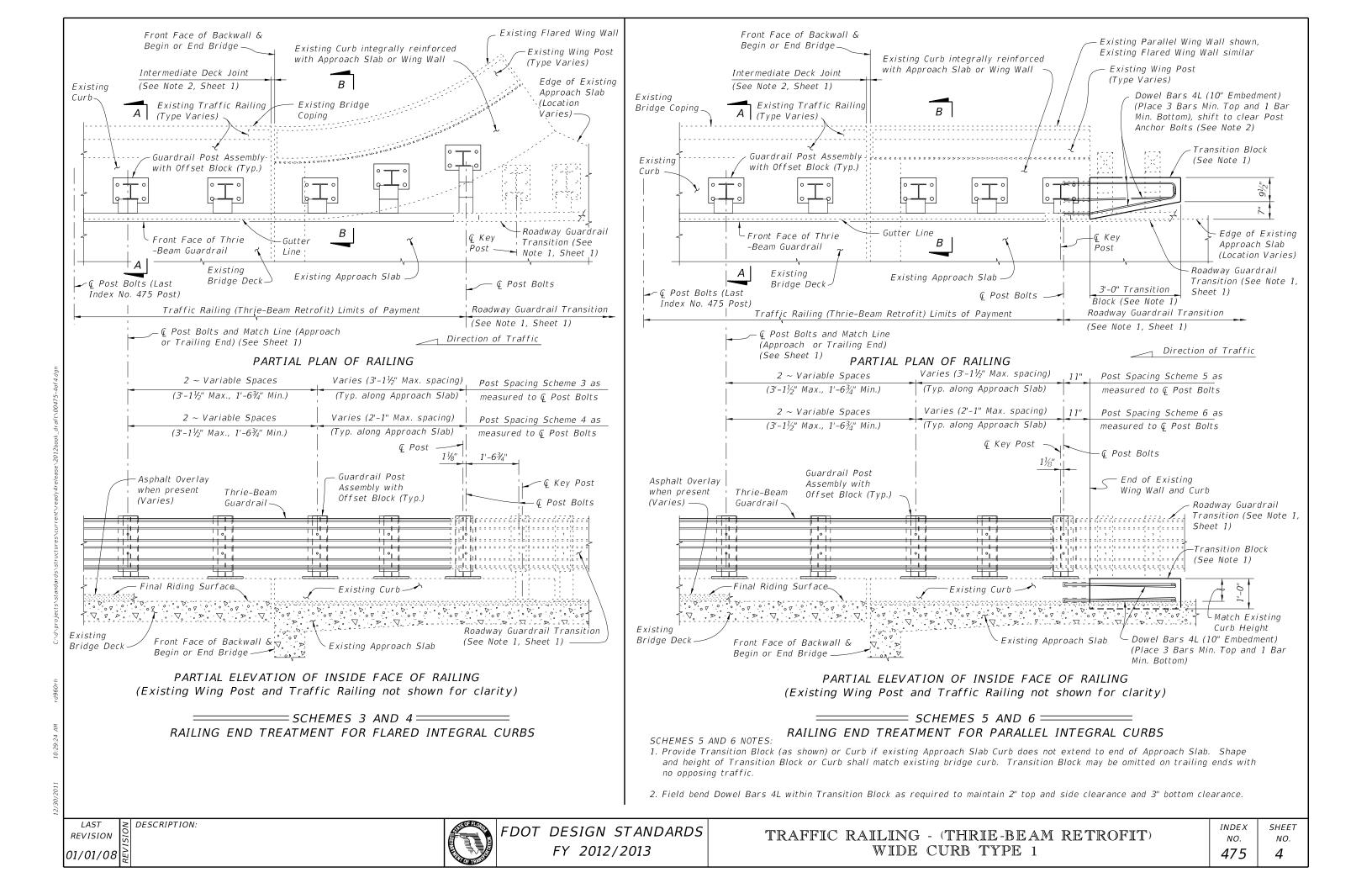


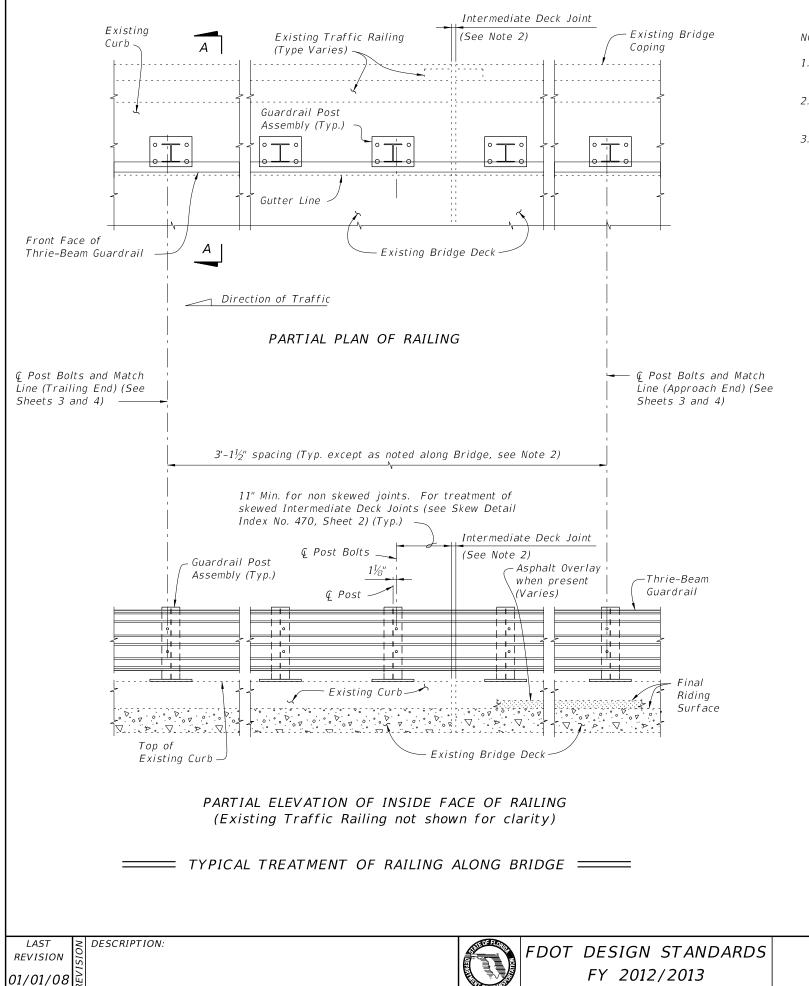
LAST REVISION









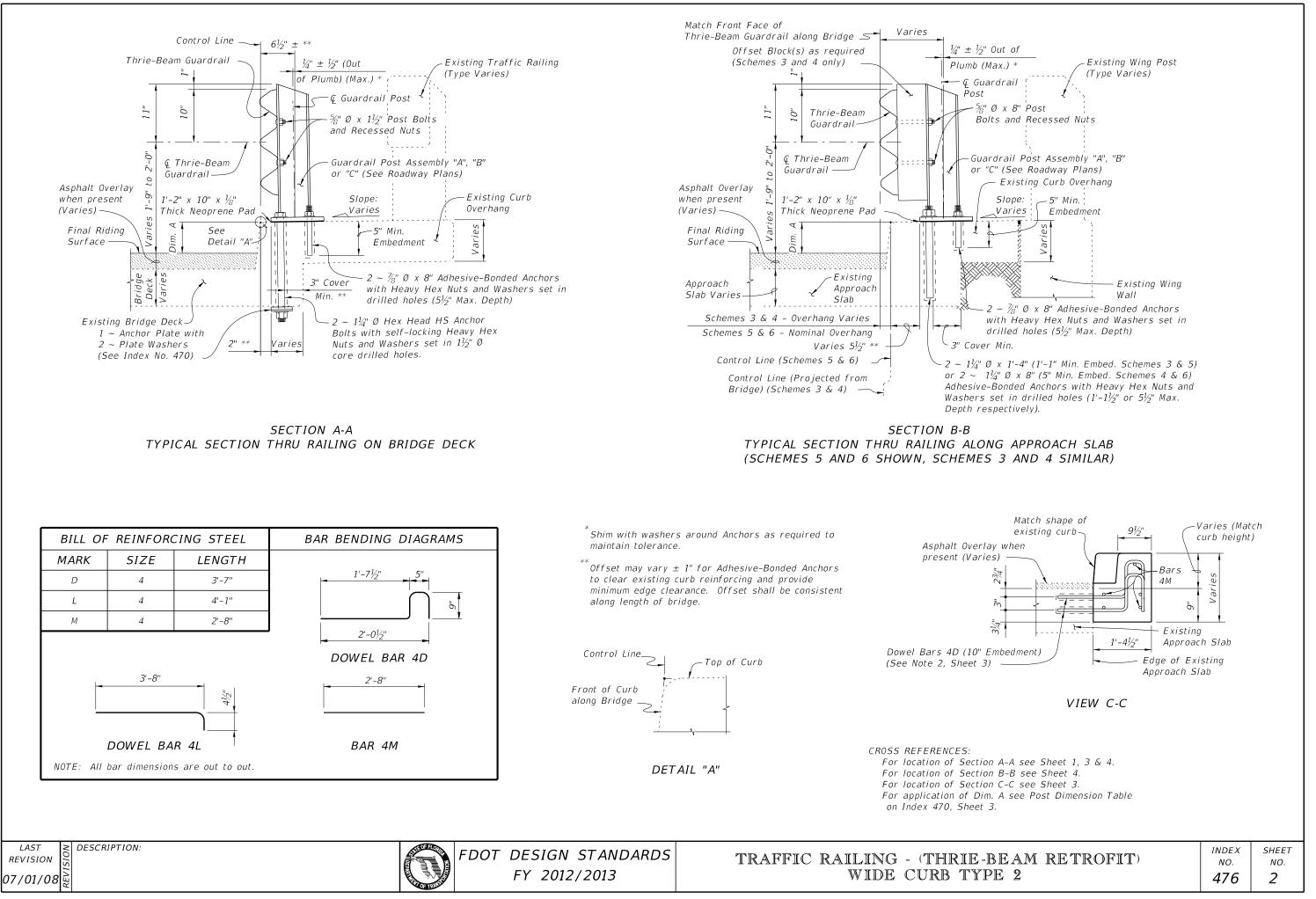


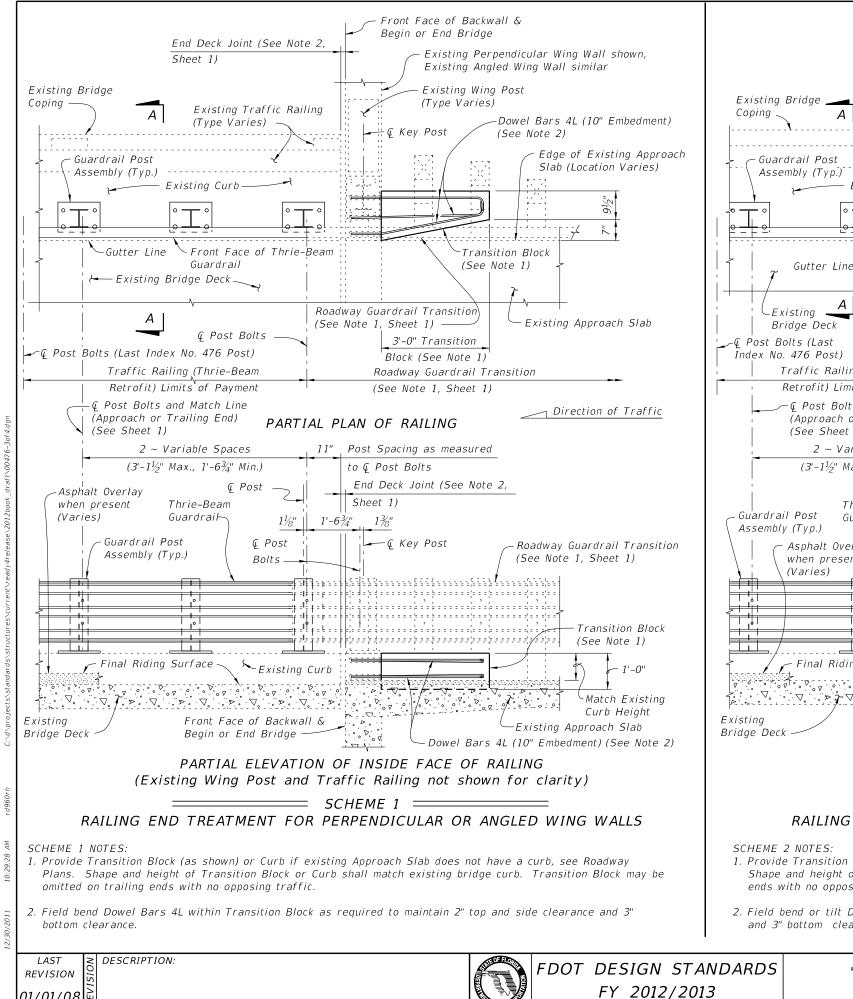
### NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

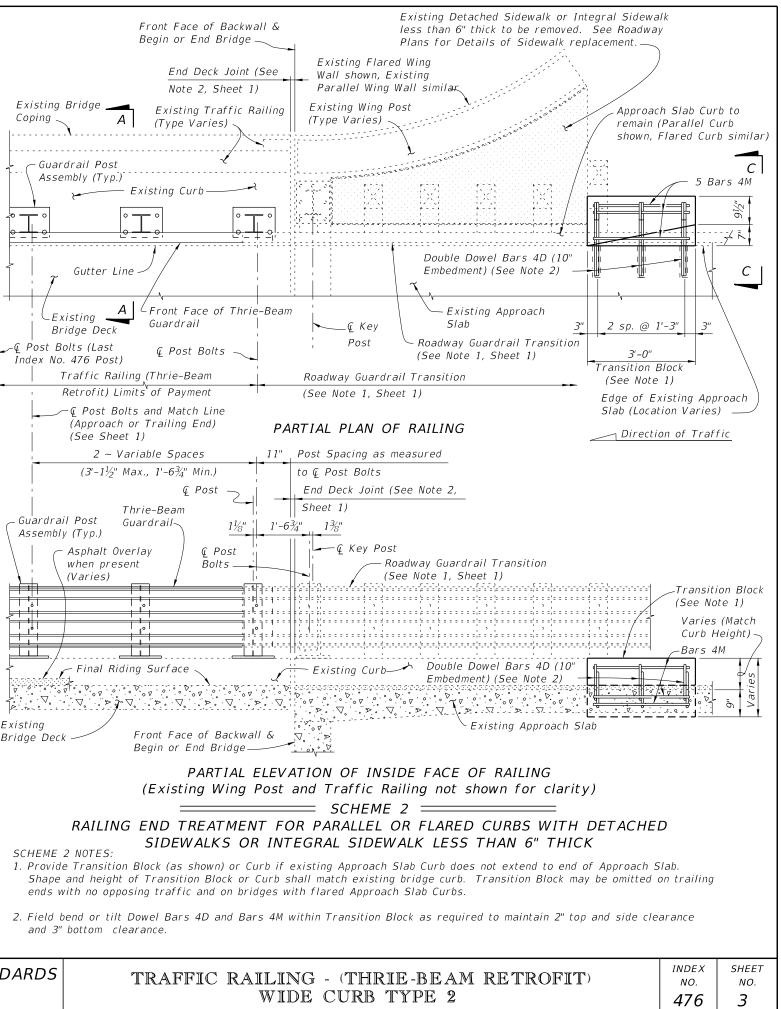
CROSS REFERENCES: For Section A-A see Sheet 2. For Traffic Railing Notes and Details see Index No. 470.

	INDEX	SHEET
TRAFFIC RAILING - (THRIE-BEAM RETROFIT)	NO.	NO.
WIDE CURB TYPE 2	476	1
		-

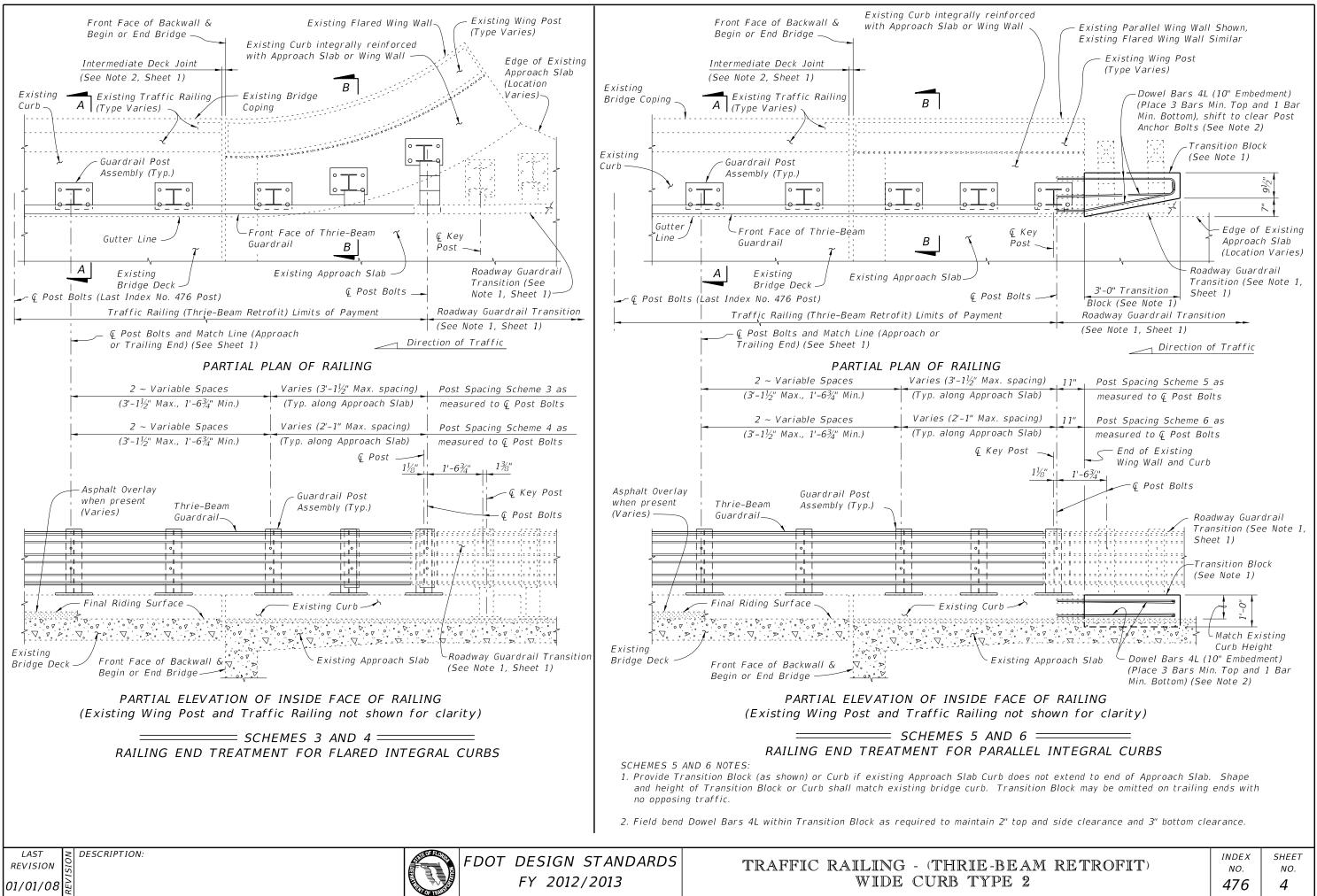




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TRAFFIC	RAILING	- (THF	RIE -
	WIDE	CURB	TYI



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### \_\_\_\_\_\_ TRAFFIC RAILING NOTES \_\_\_\_\_\_

This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested previously and approved for a NCHRP Report 350 Test Level 4 rating, except for the Tapered End Transition on Index No. 484.

CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.

EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment). BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments.

Details for bridges on horizontally curved alignments are similar.

NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or  $\frac{3}{6}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures.

ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The Department will determine the vertical Datum information for the marker.

SURFACE FINISH: Unless otherwise shown in the Plans, place a Class 5 Applied Finish Coating on the top and sides of the Traffic Railing (Vertical Face Retrofit).

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table below. Reflector color (white or yellow) shall match the color of the near edgeline.

PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) include all materials and labor required to construct the railing. Incidental work as required for transition blocks, curbs, spread footings approaches, reflective railing markers (including installation) shall also be included under Traffic Railing (Vertical Face Retofit).

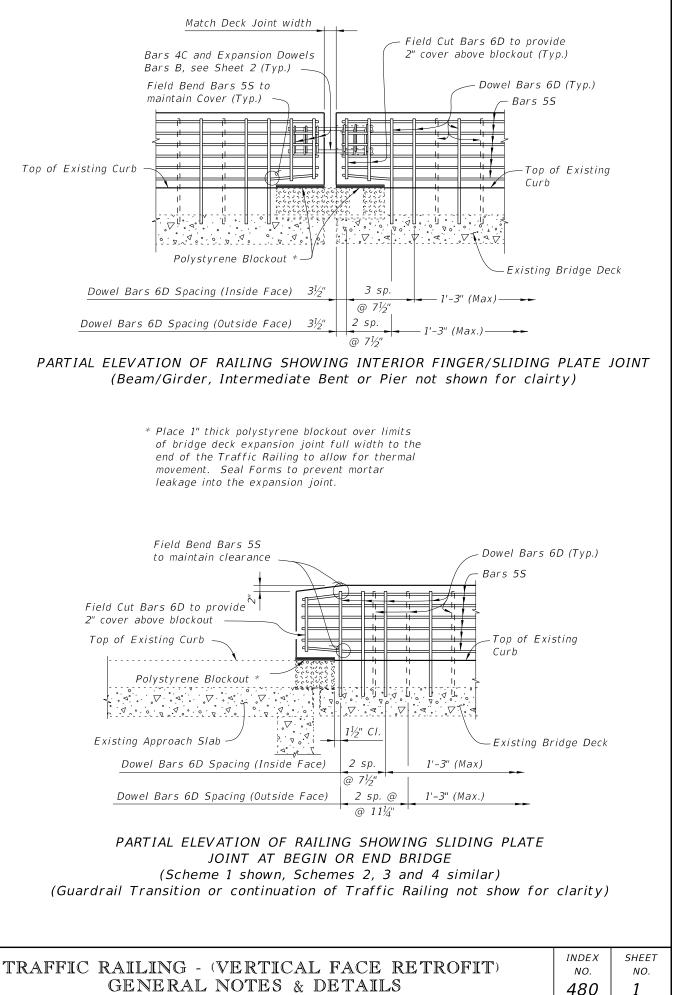
Limiting Station of Transition	1'-0''	Varie	5	J	
(See Roadway Plans)	(Min.)	(2'-6" №	lin.)		
م NAME OR DAT NAME OR DAT BRIDGE NUMBL Top of Existing Curb	ĒR				
NAME, DATE AND BI	, ∇ , , , , , , , , , , , , , , , , , ,	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	₽		

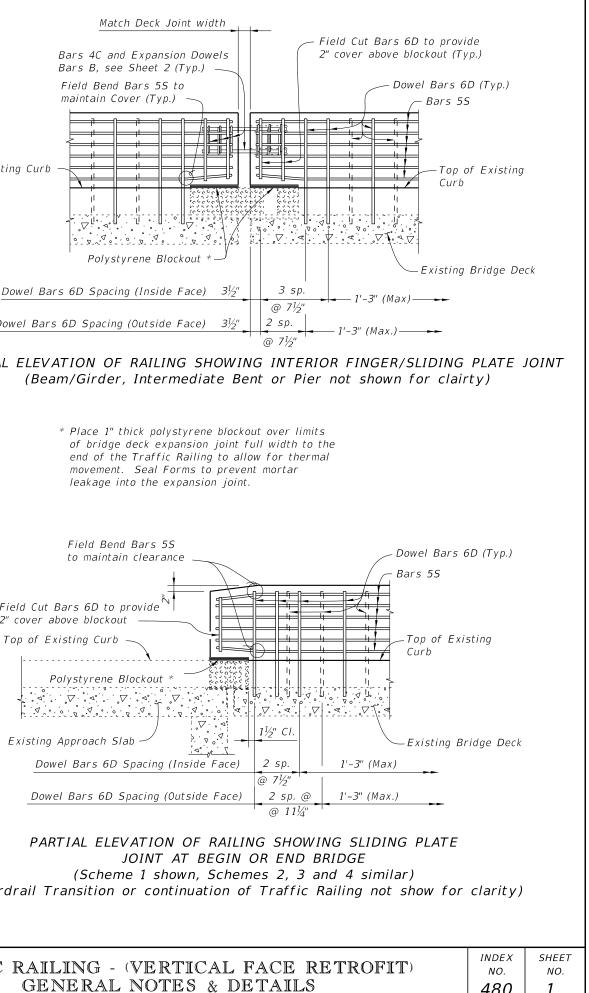
LETTERING DETAIL

ESTIMATED TRAFFIC RAILING QUANTITIES				
ITEM	UNIT	QUANTITY		
ITEM UNI		9" Curb	Increment	
Concrete	CY/FT	0.064	0.003 per in. height	
Reinforcing Steel	LB/FT	13.27	0.10 per in. length	

REFLECTIVE RAILING MARKER SPACING	
Distance – Edge of Travel Lane to Face of Railing	Spacing (Ft.)
< 4'	40'
4' to 8'	80'
> than 8'	None Required

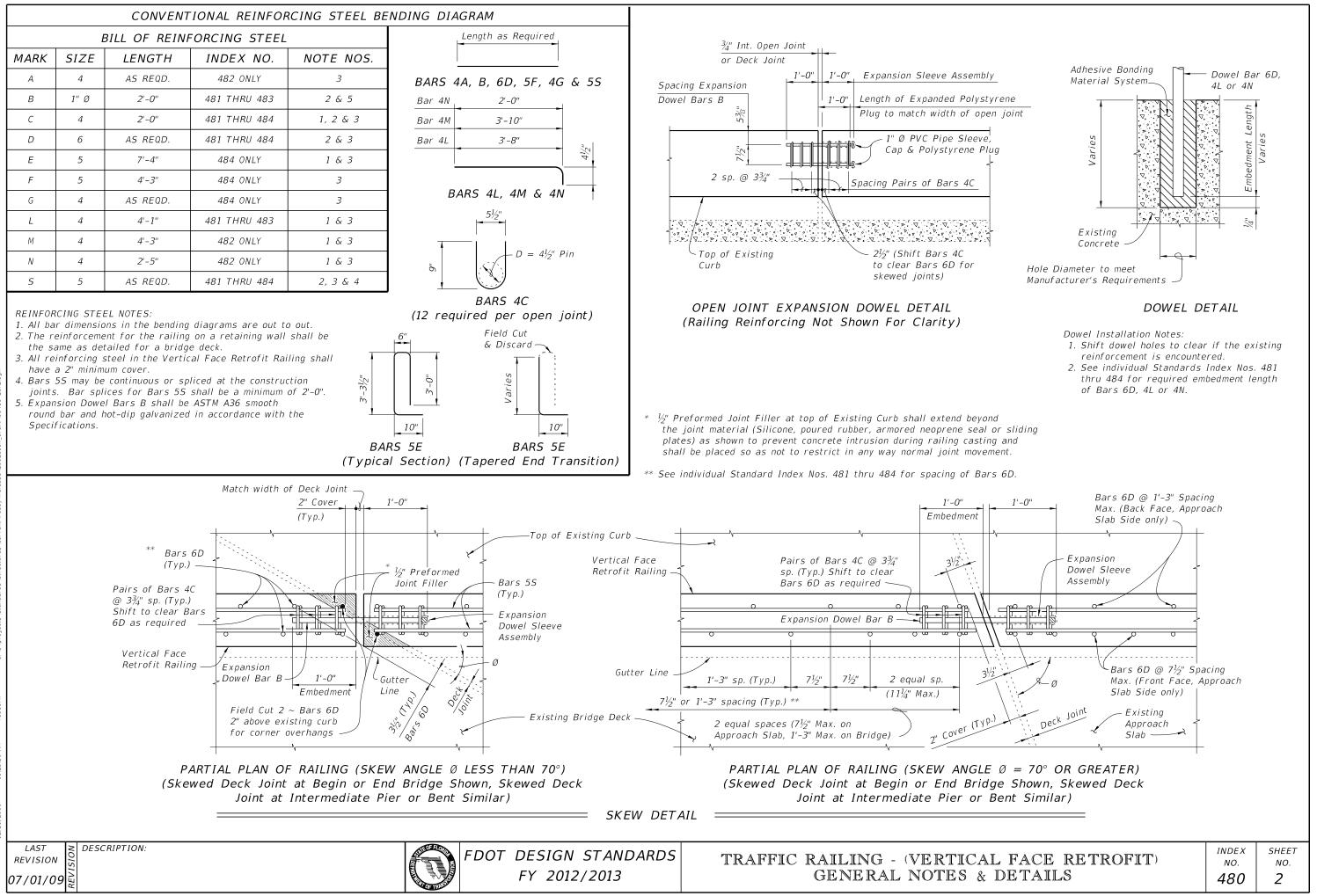
(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index No. 484, Sheet 4 for Spread Footing Approach Quantities.

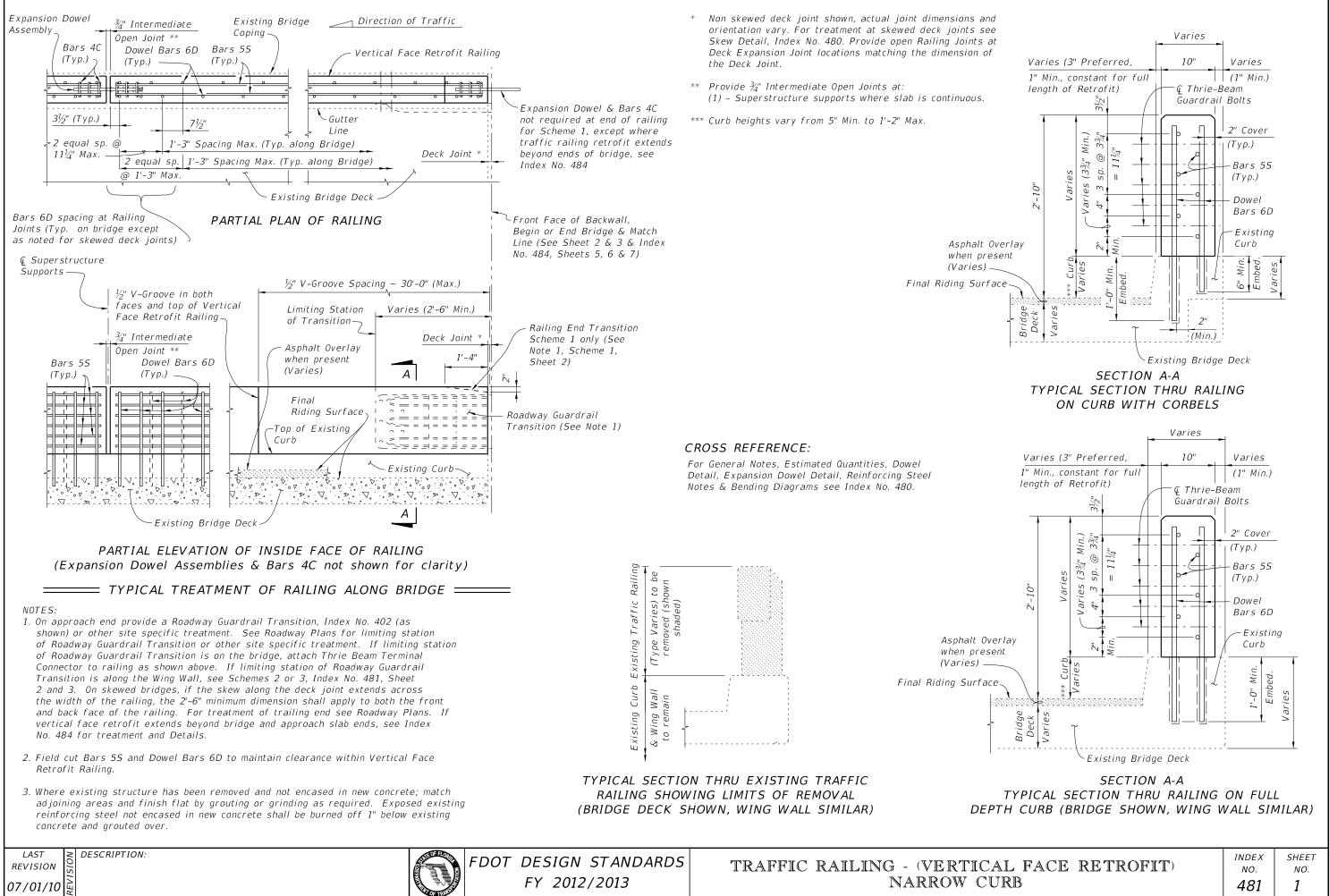


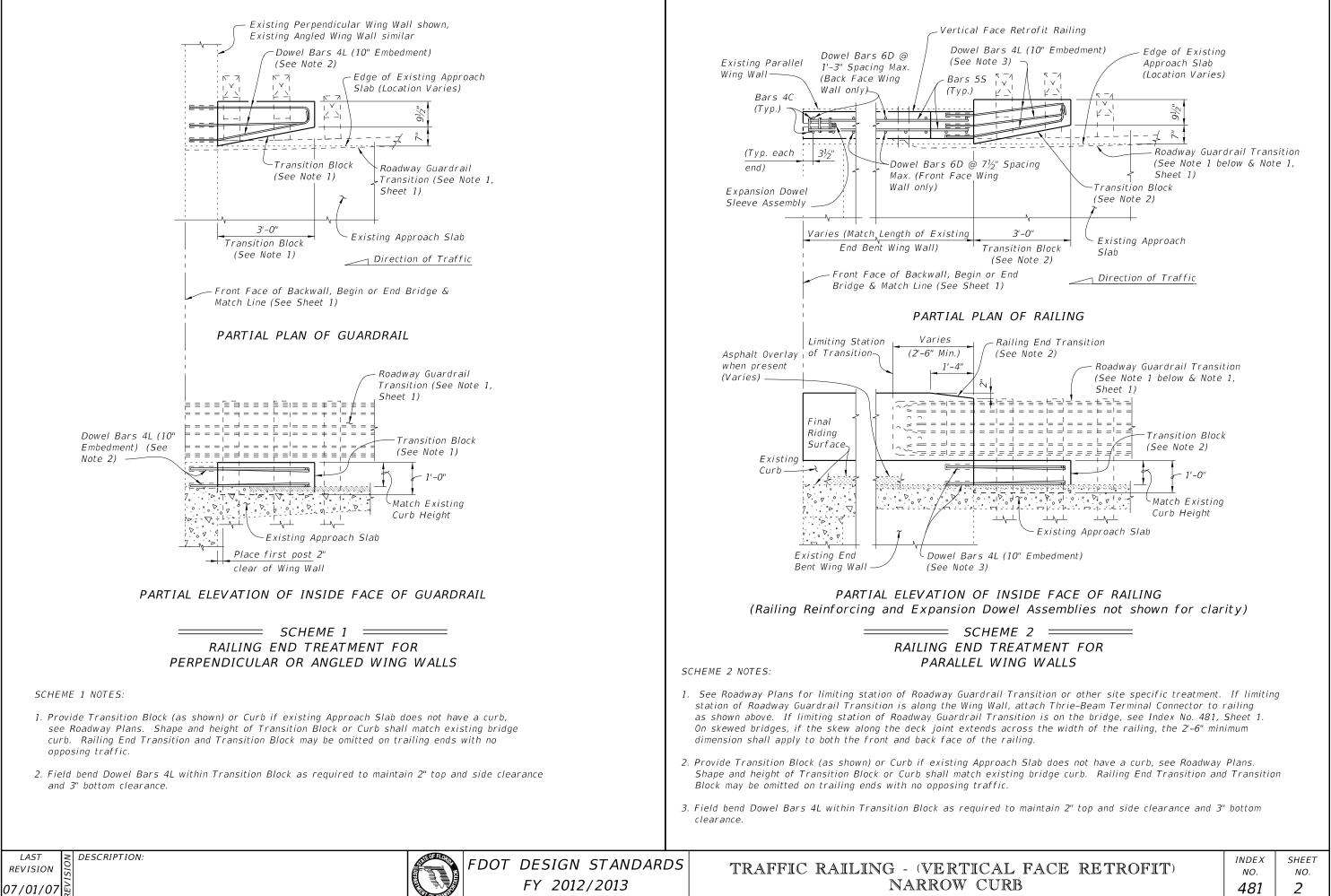


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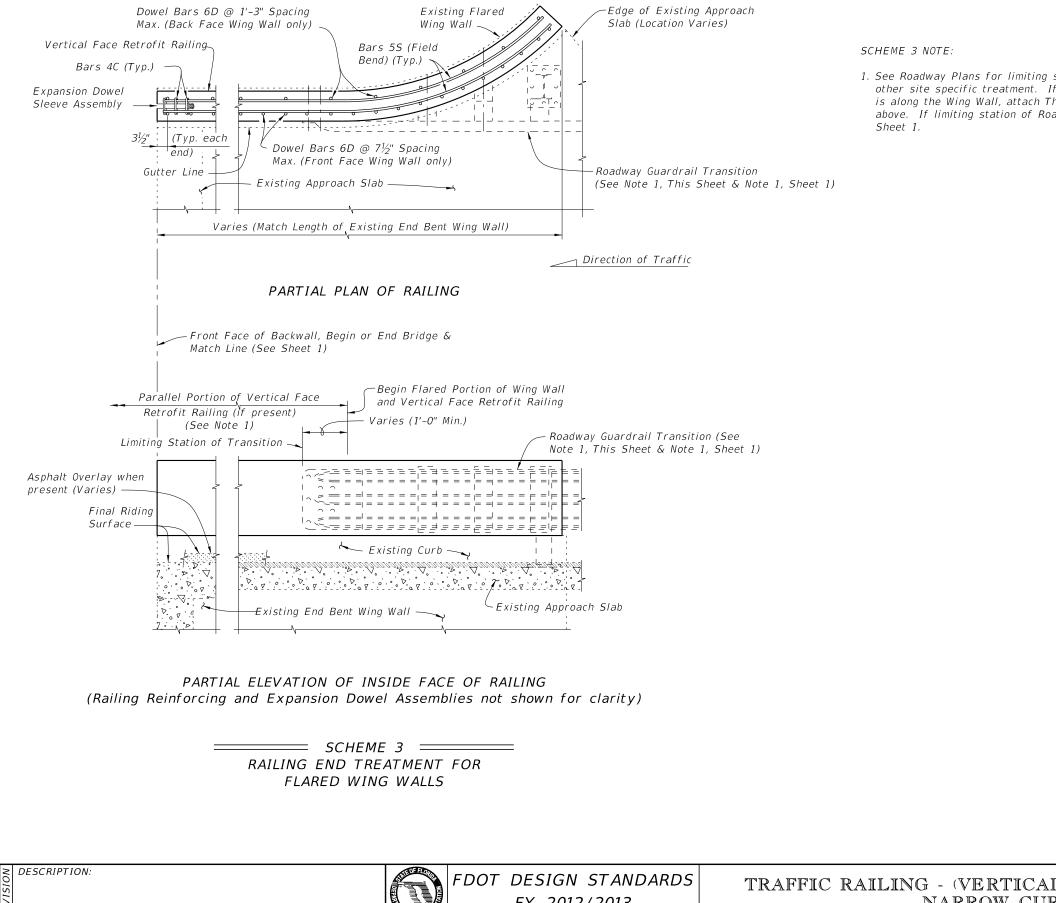








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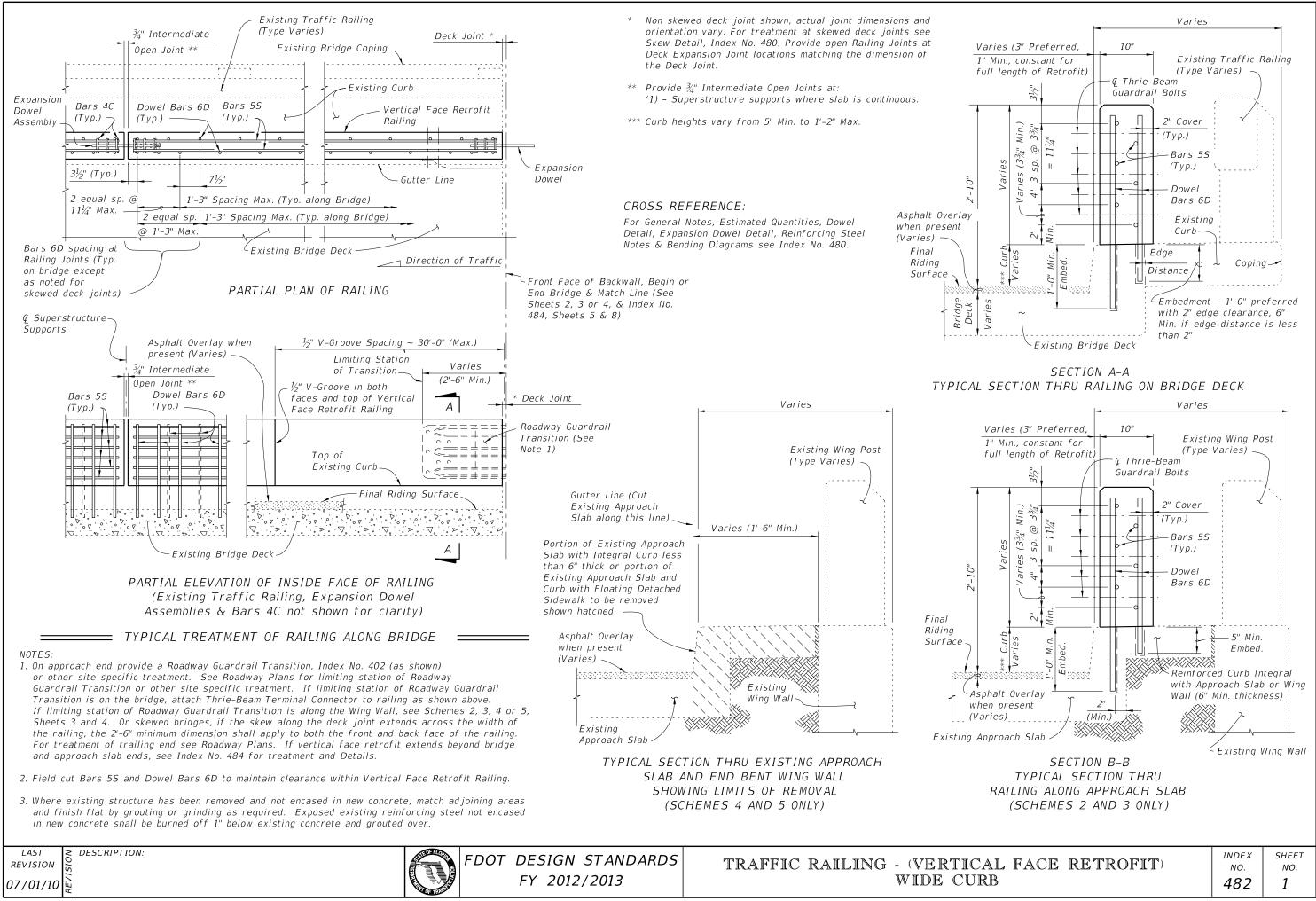
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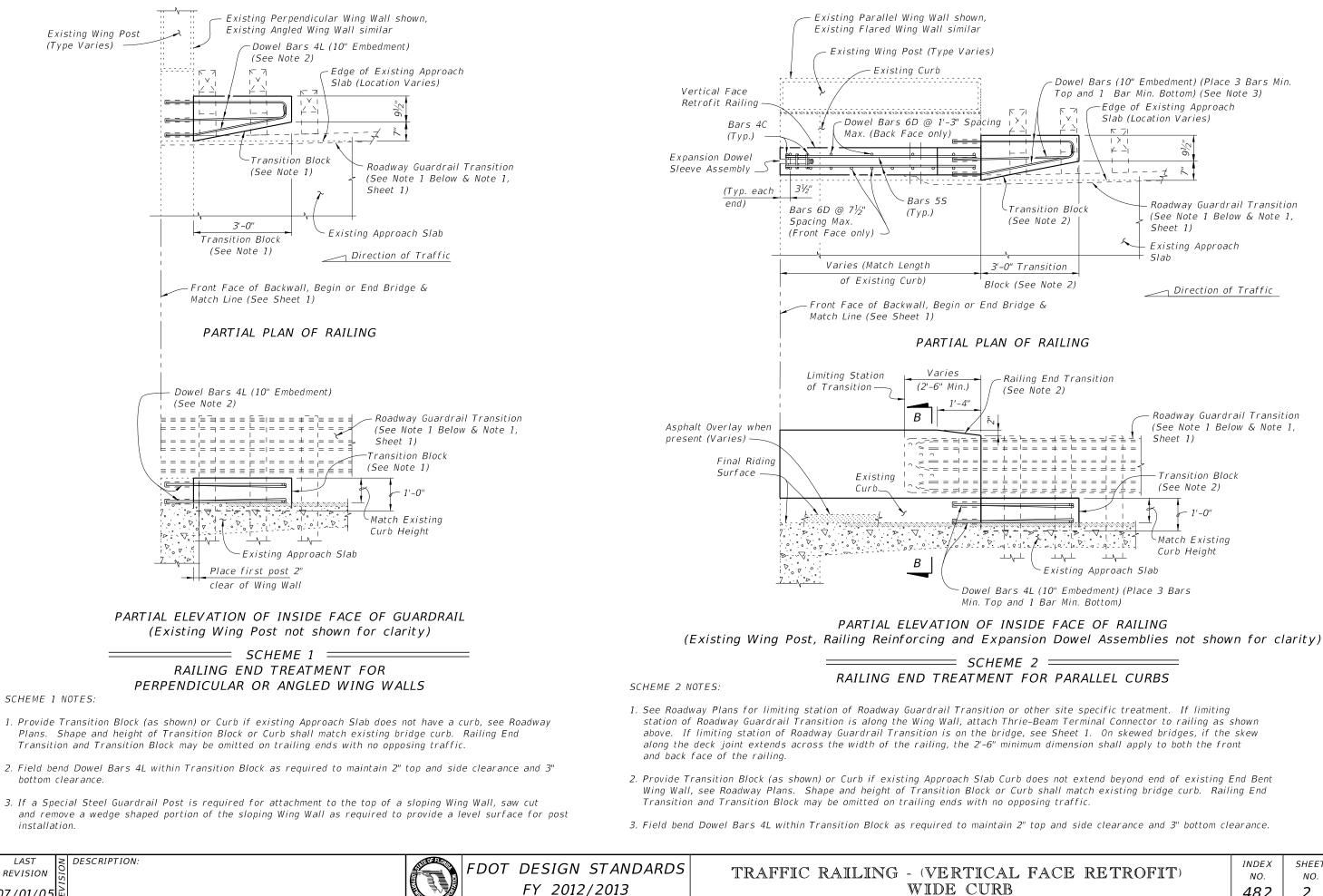
NARROW CUR

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see

L FACE RETROFIT) RB	index NO. <b>481</b>	sнеет NO. <b>3</b>

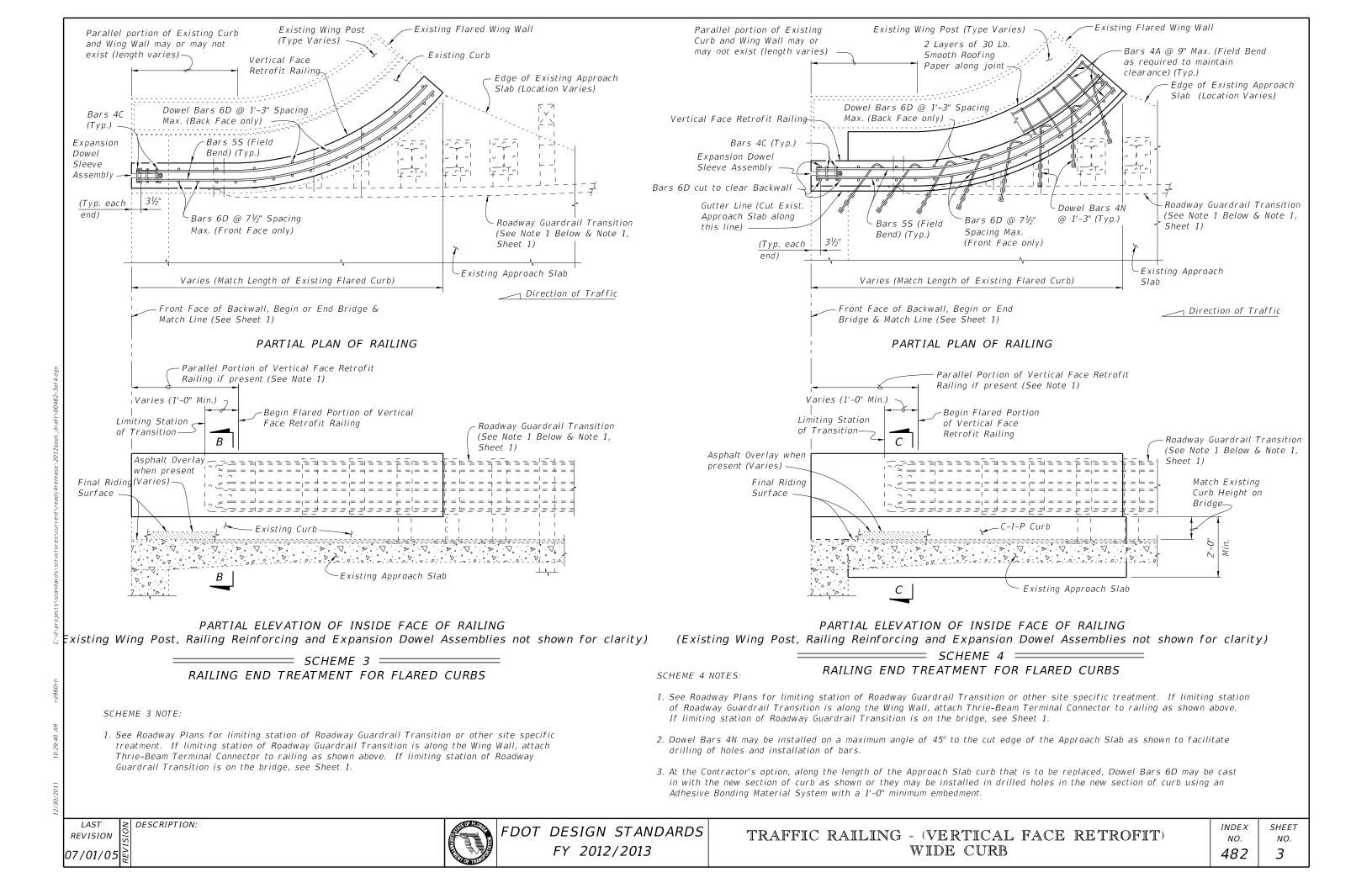


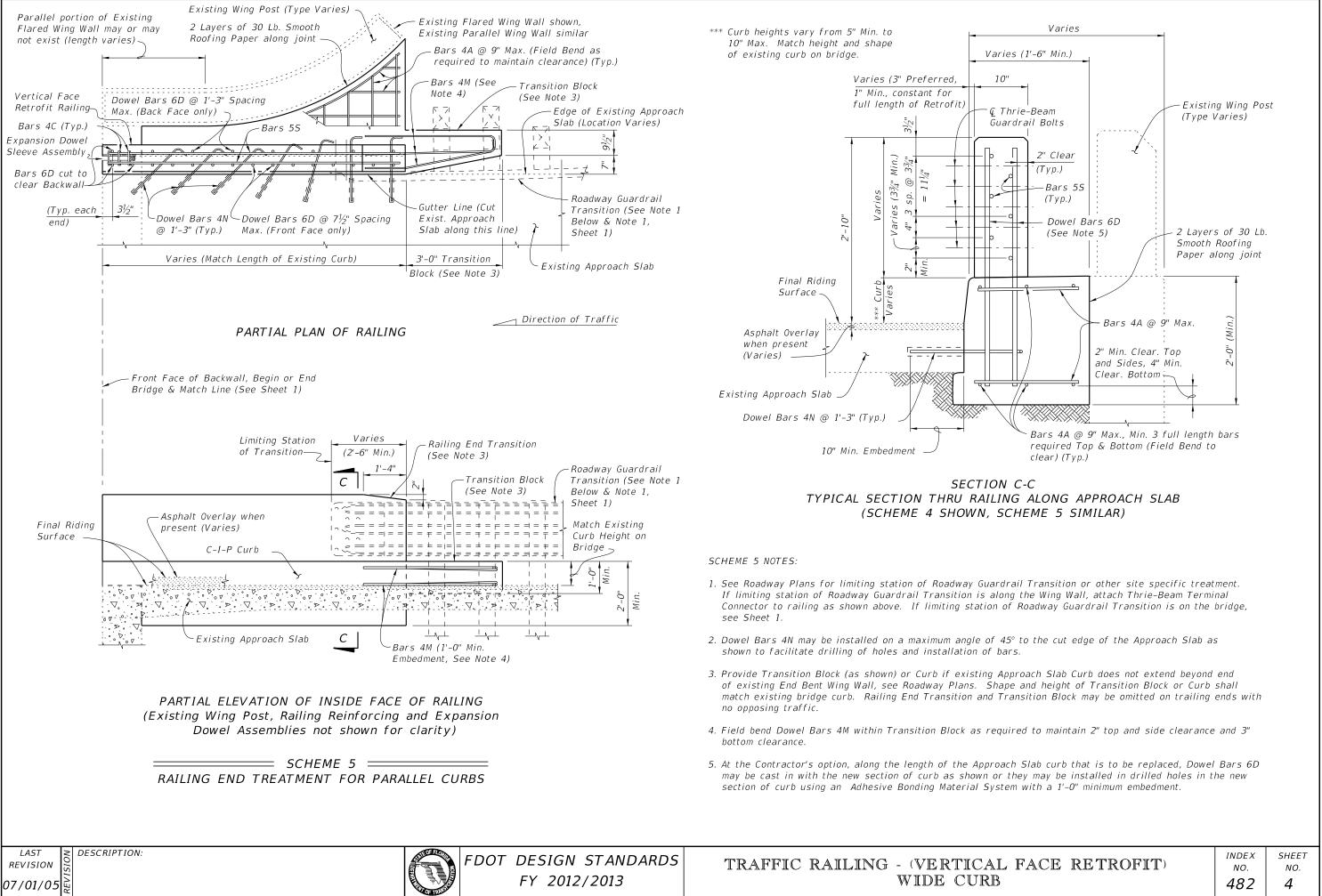
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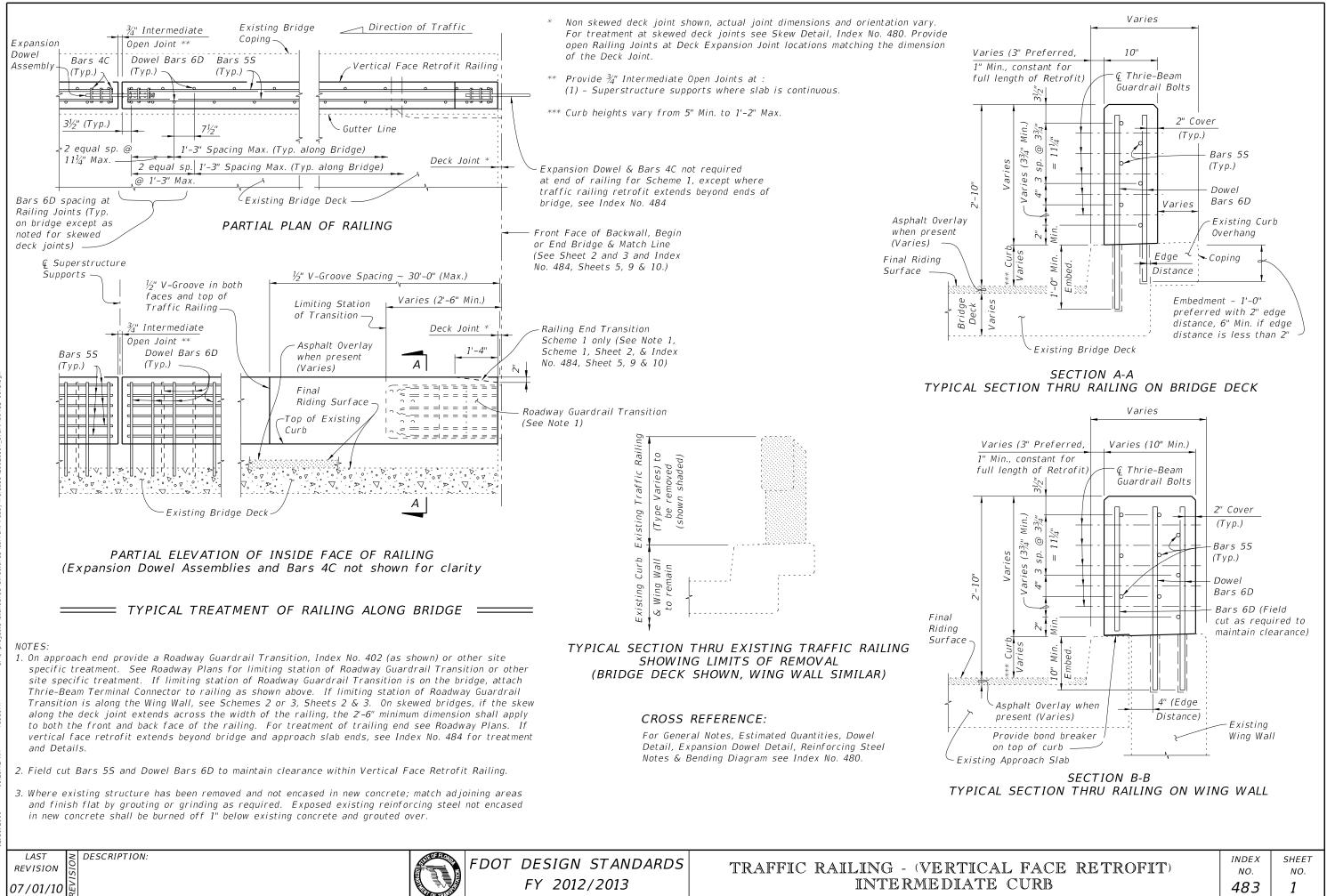


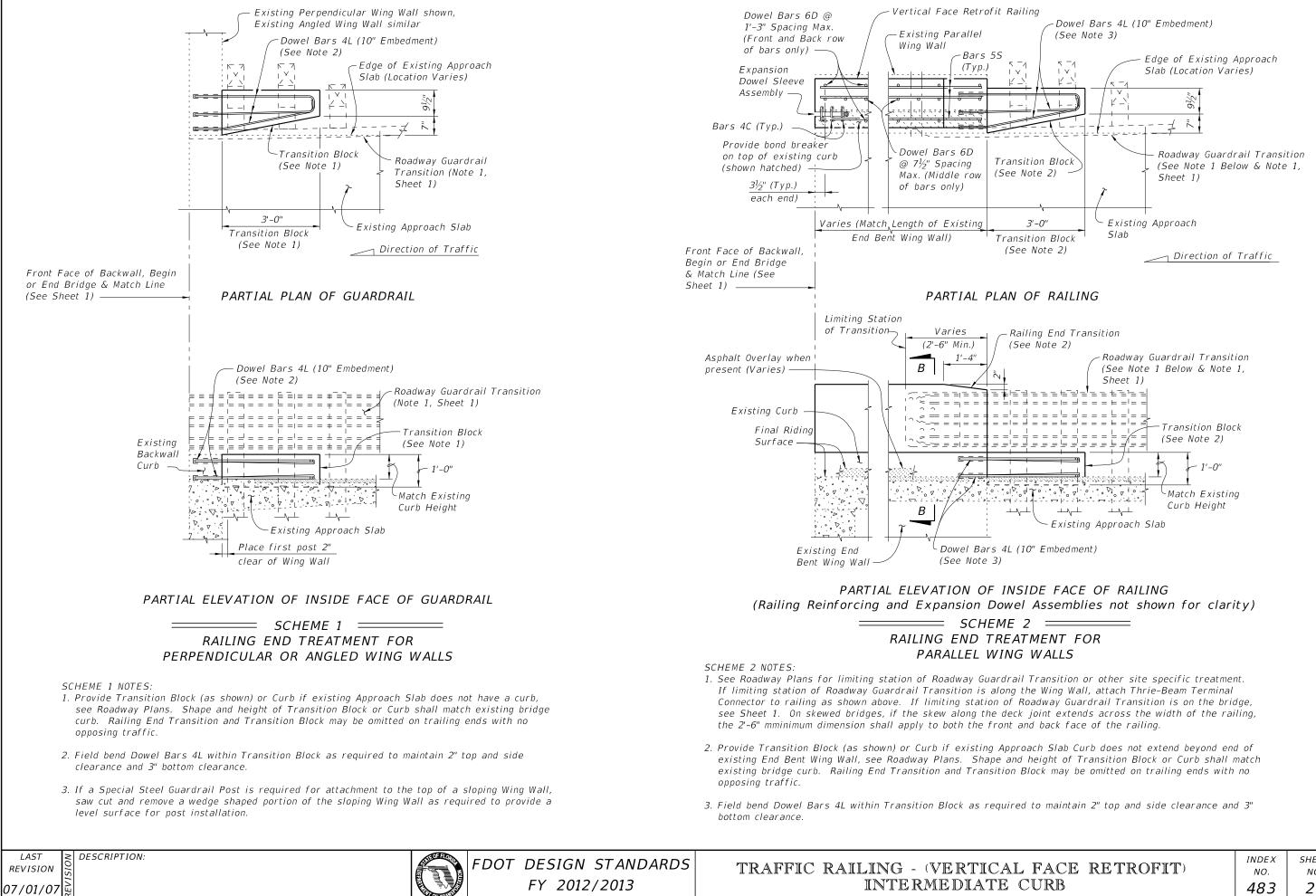
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L	FACE	RETROFIT)	INDEX NO.	SHEET NO.
6			482	2

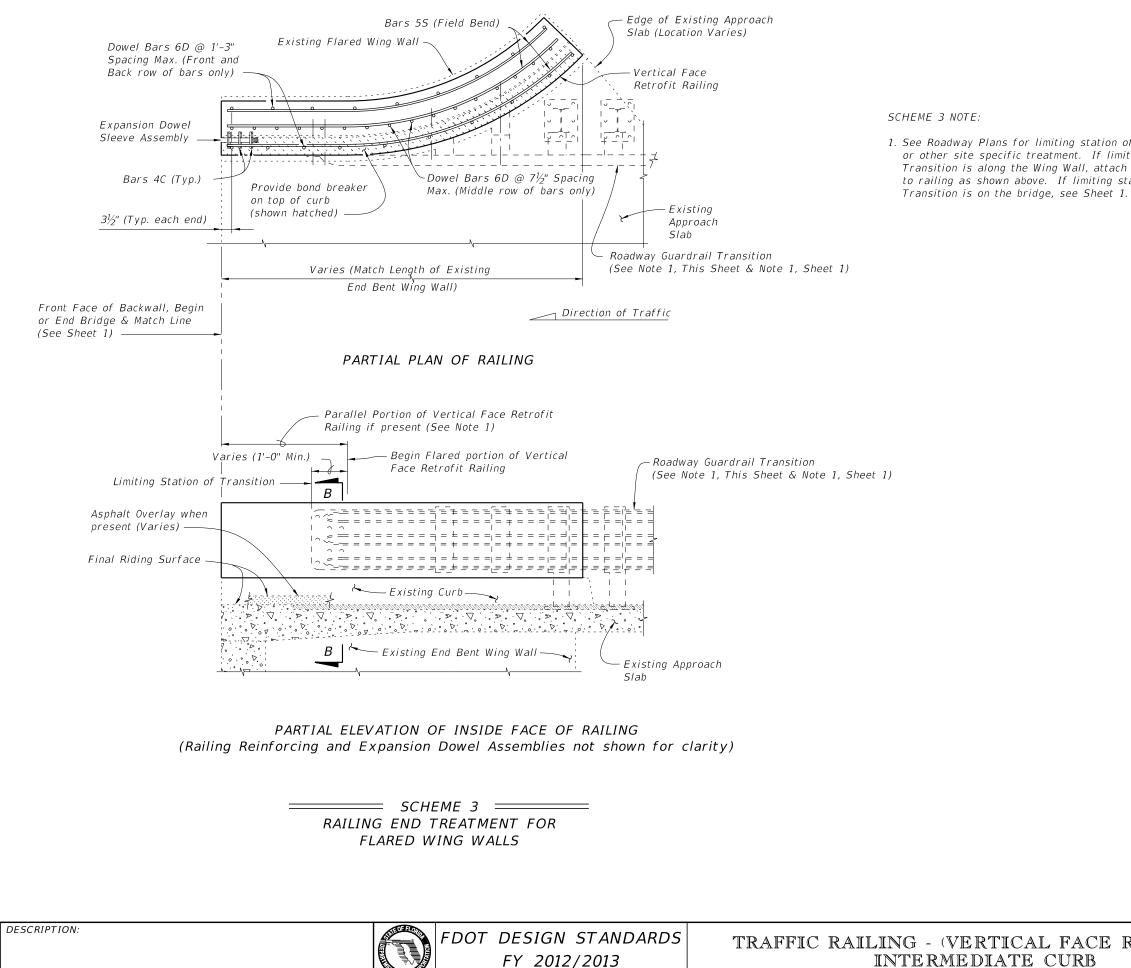








L FACE RETROFIT	INDEX NO.	SHEET NO.
CURB	483	2

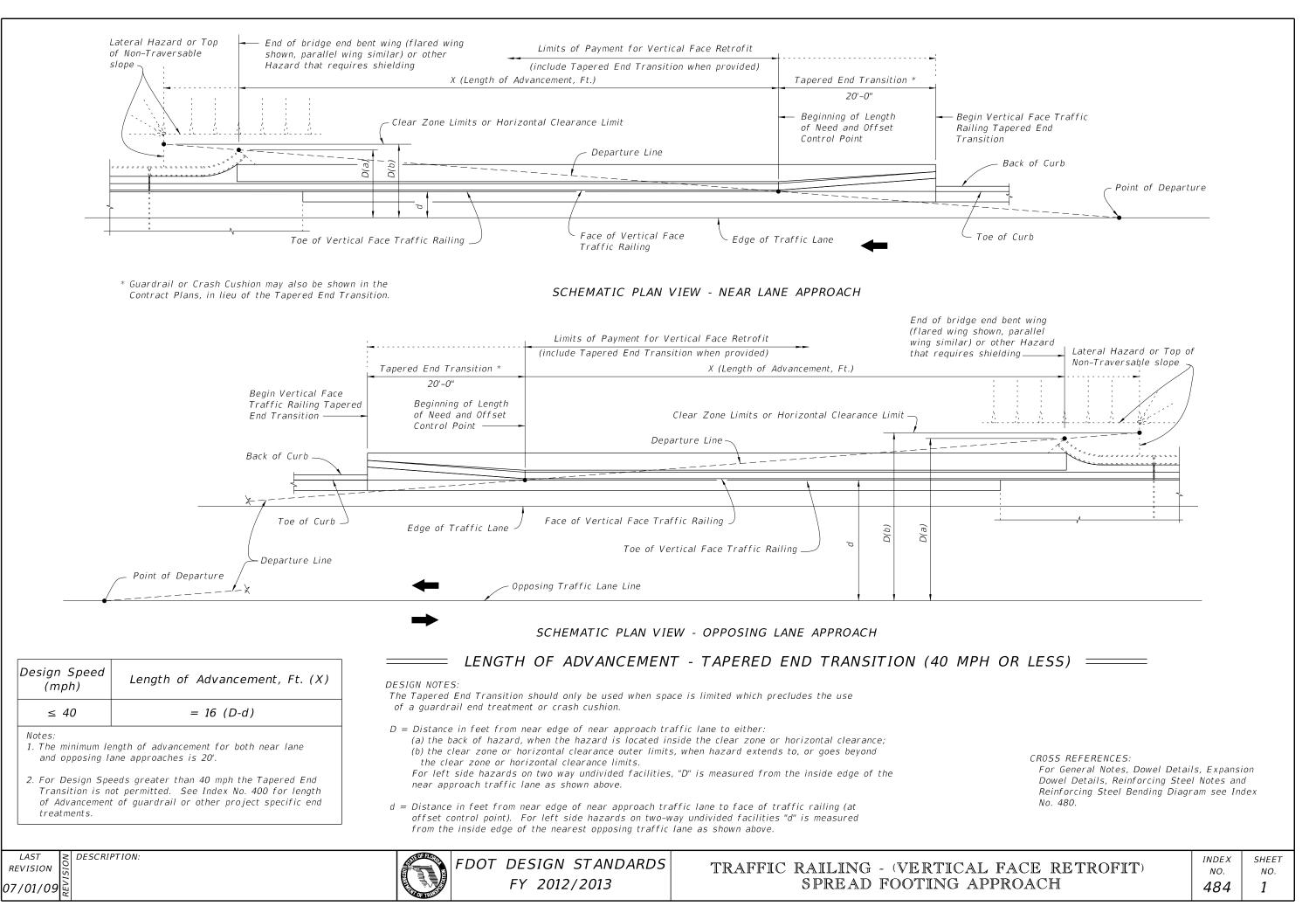


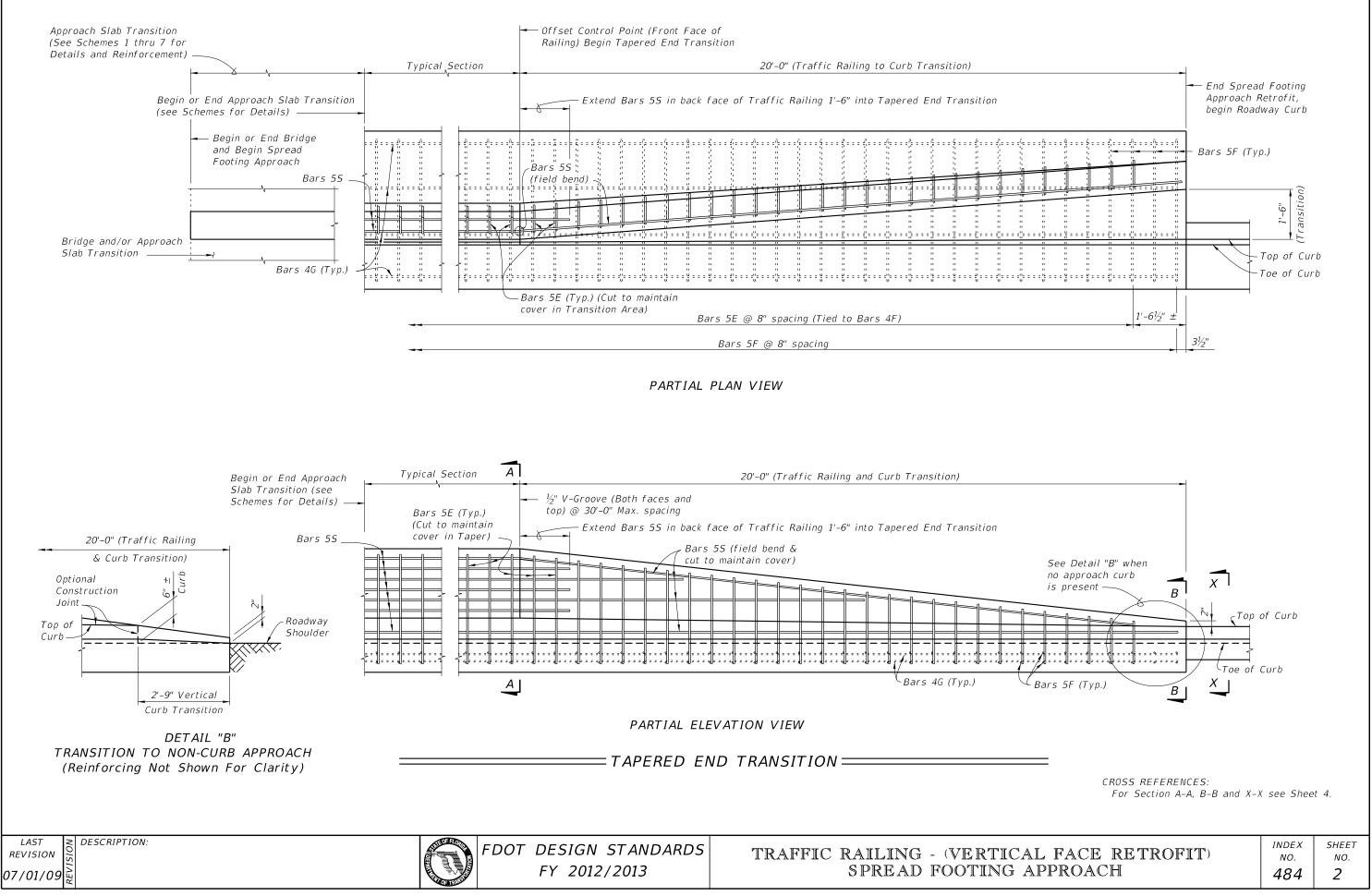
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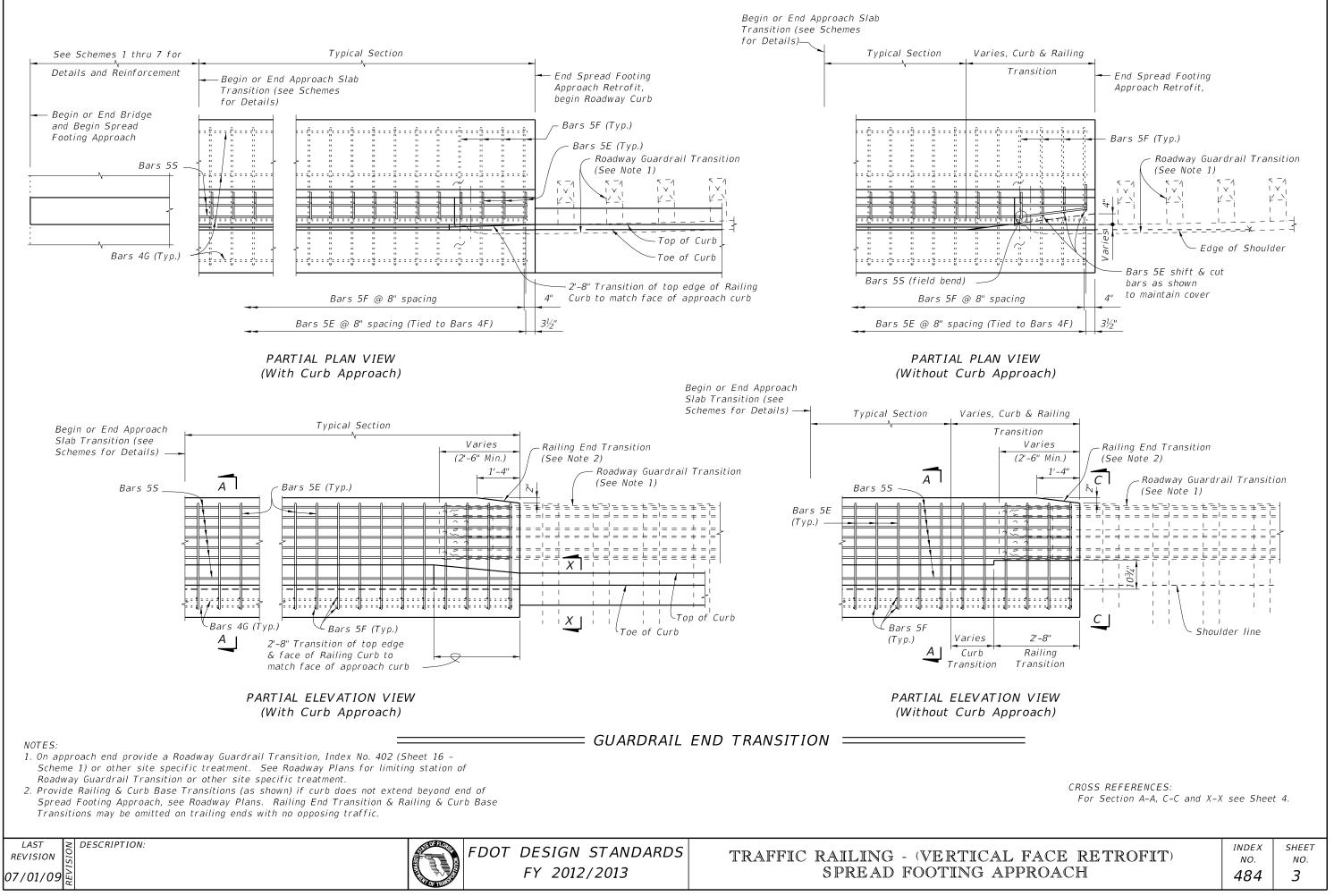
1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail

L FACE	RETROFIT)	INDEX NO.	SHEET NO.
CURB		483	3

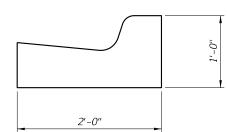




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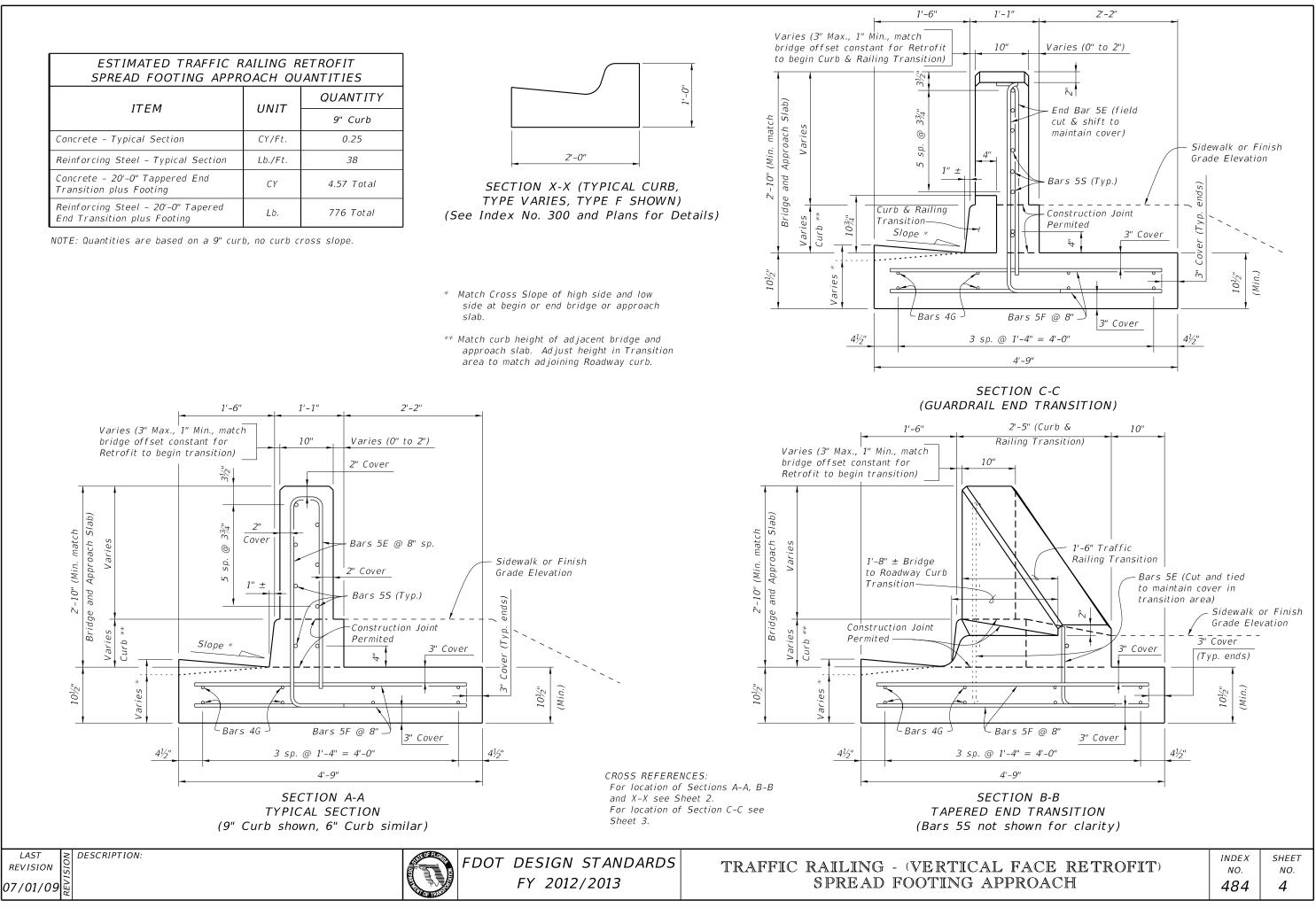


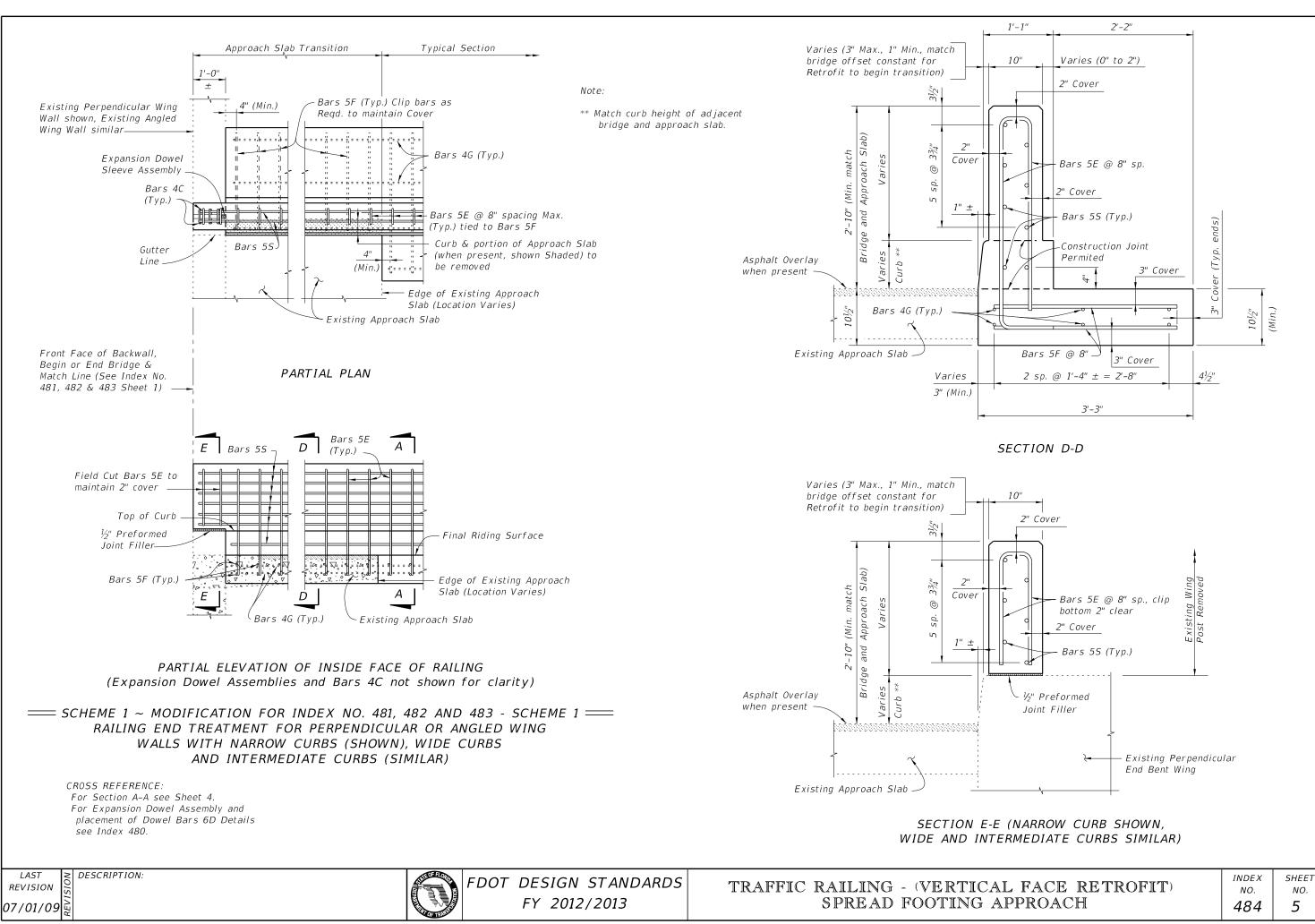
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES			
ITFM	UNIT	QUANTITY	
	UNIT	9" Curb	
Concrete – Typical Section	CY/Ft.	0.25	
Reinforcing Steel - Typical Section	Lb./Ft.	38	
Concrete - 20'-0" Tappered End Transition plus Footing	СҮ	4.57 Total	
Reinforcing Steel - 20'-0" Tapered End Transition plus Footing	Lb.	776 Total	



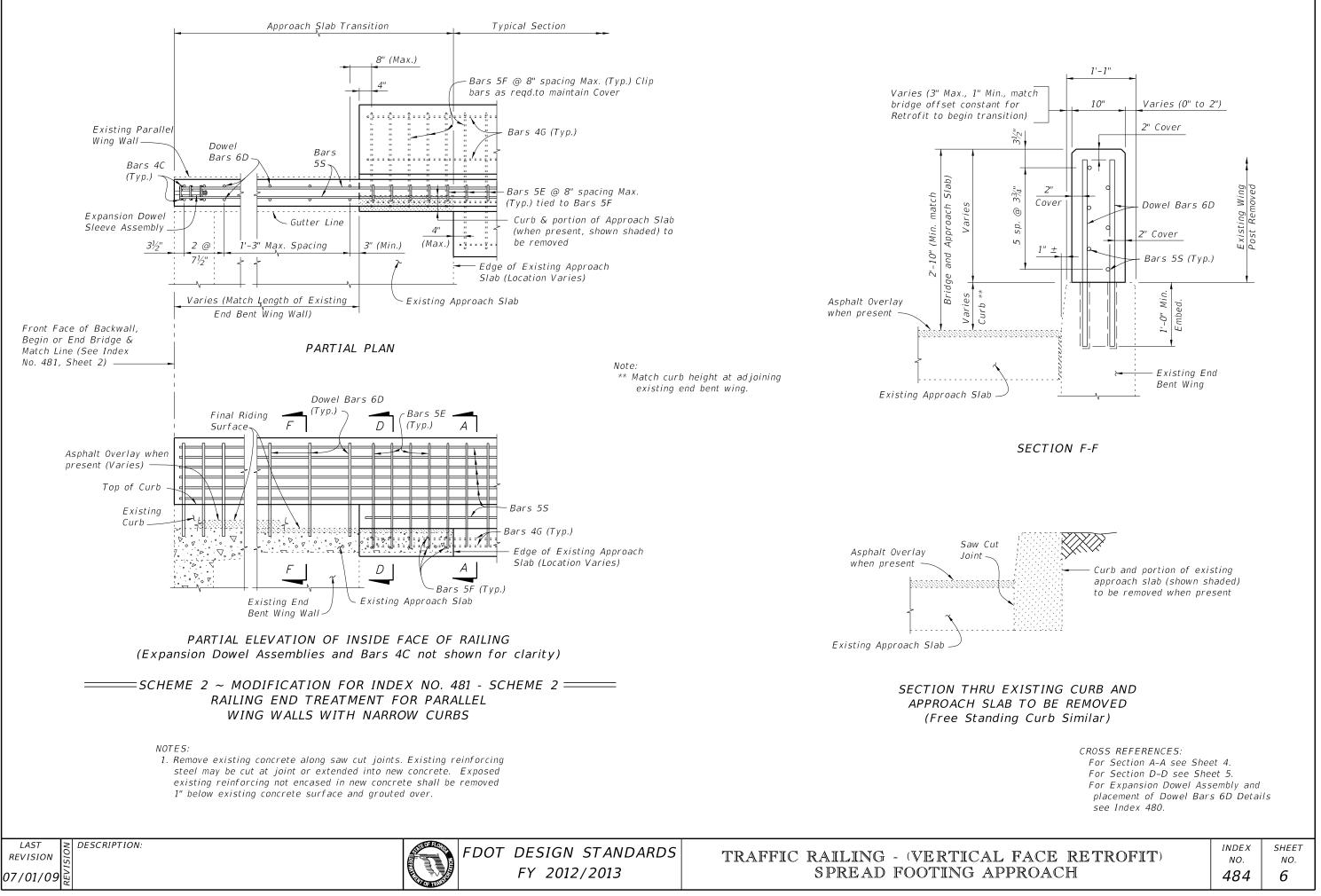
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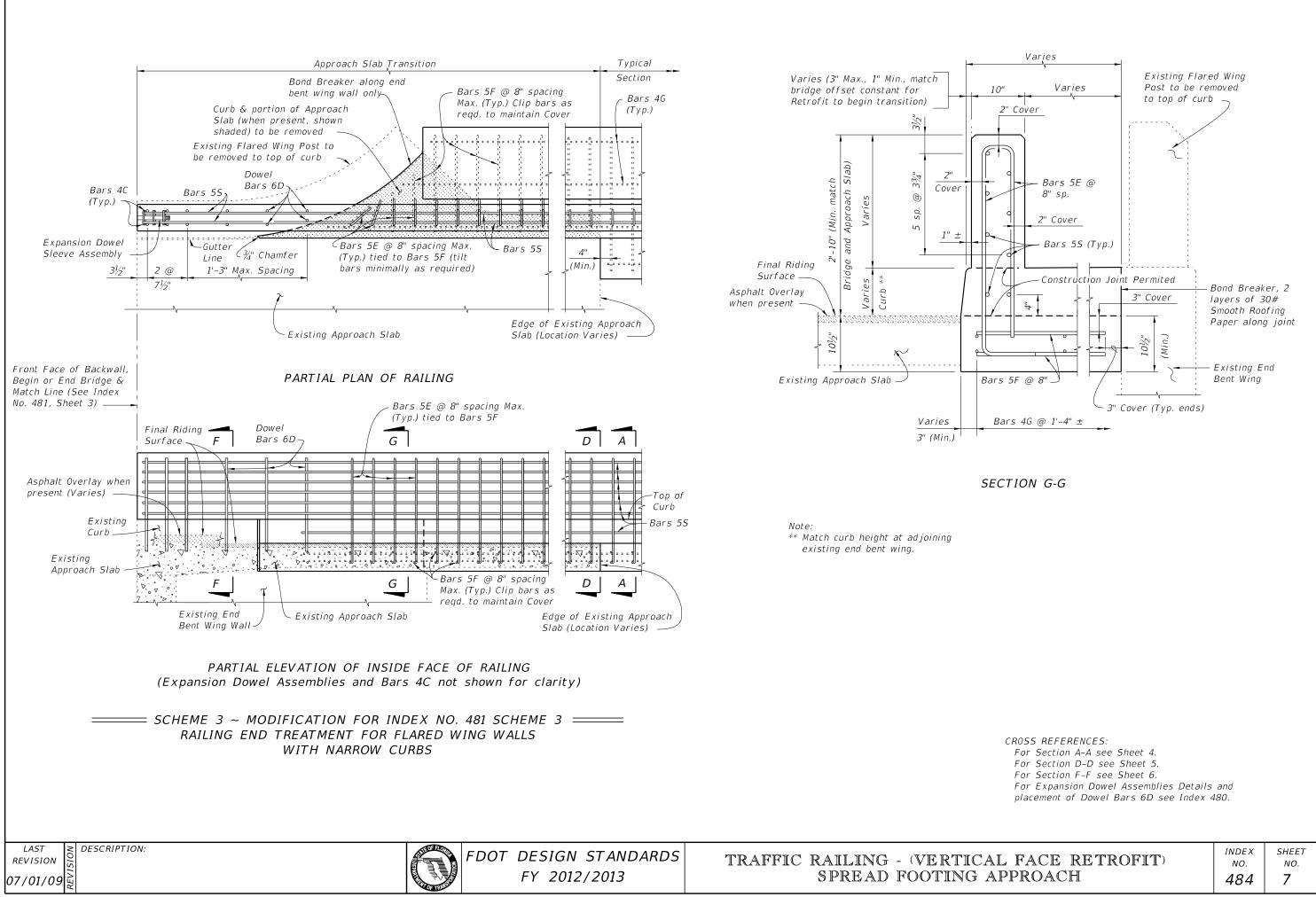
bridge offset constant for Retrofit to begin Curb & Railing Transition) " (Min. match Approach Slab) 0 Vari sp. 4" 1" ± 2'-10" e and , Bridge Curb & Railing 10%es Transition-Curb Slope 01/2 Varies -Bars 4G -4½"

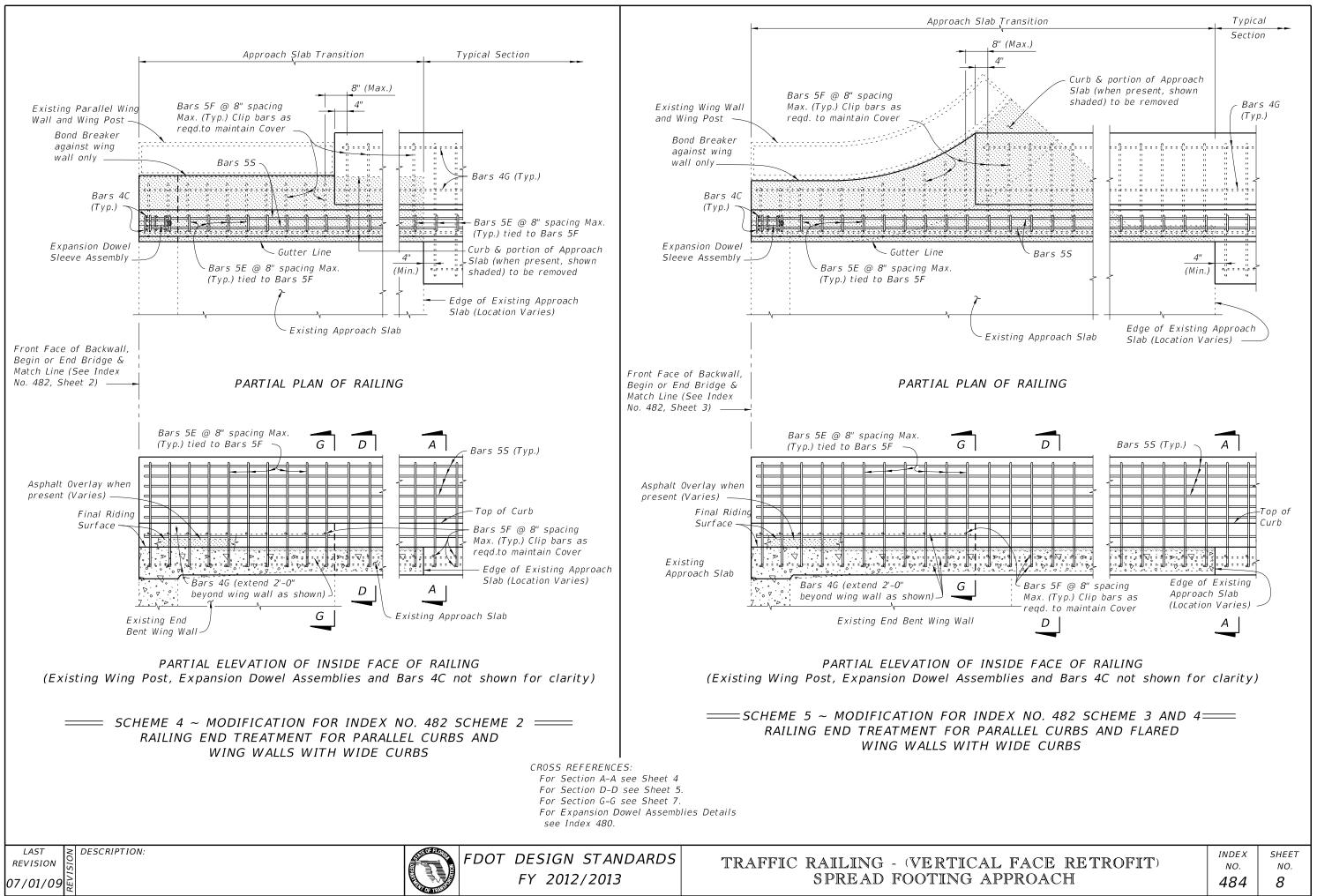




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