DESIGN LOADS, GEOMETRY AND APPLICABILITY:
See the Instructions for Design Standards for the design loads, geometry and applicability requirements.
GENERAL
GENERAL:
Adequate foundation support shall be provided for anchorage and stability against overturning. See Index No. 85 for special requirements and modifications for use on bridges. The railing shown on these drawing requires a
handrail for ramps steeper than a $5 \%$ grade to conform with the requirements of the Americans with Disabilities
ACt (ADA).
RAILS, PANELS AND POSTS
Pipe Rails and Pickets shall be in accordance with ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) or ASTM A36 for bars. Structural Tube shall be in accordance with ASTM A500 Grade A, B, C or D, or ASTM A501. Perforated panels (Type 5), U-Channels and filler plates shall be ASTM A36 or A1011
(Grade 36). Posts and End Rails shall be fabricated and installed plumb, $\pm 1^{\prime \prime}$ tolerance when measured at $3^{3}-6^{\prime \prime}$ Grade 36). Posts and End Rails shall be fabricated and installed plumb, $\pm 1$ tolerance when measured at the 2, $3 \& 5$ panel infills may be fabricated parallel to the longitudinal grade. Corners and changes in tangential longitudinal alignment shall be made continuous with a $9^{\prime \prime}$ bend radius or terminate at ad joining sections with mitered
end sections when handrails are not required. For changes in tangential longitudinal alignment greater than $45^{\circ}$ end sections when handrails are not required. For changes in tangential longitudinal alignment greater than 45",
posts shall be positioned at a maximum distance of $2^{\prime}-0^{\prime \prime}$ each side of the corner and shall not be located at the corner apex. For curved longitudinal alignments the top and bottom rails and handrails shall be shop bent to match the alignment radius.
BASE PLATES AND RAIL CAPS:
Base Plates and Rail Cap Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36
Shim Plates shall be aluminum in accordance with ASTM B209, Alloy 6061 or 6063 . Shim plates shall be used oundation height adjustments greater than $1 / / 1 /$ and localized irregularities greater than $1 / /{ }^{\prime \prime}$. Field trim shim
plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of
trimmed flat shim plates shown. Stacked shim plates must be bonded together with adhesive bonding material and limited to a maximum total thickness of $1 / 2$ ", unless longer anchor bolts are provided for the exposed thread length.
ANCHOR BOLTS:
Anchor bolts shall be in accordance with ASTM F1554 Grade 36. Headless anchor bolts for Adhesive Anchors shall be threaded full length. Cutting of reinforcing steel is permitted for drilled hole installation. Expansion Anchors are not permitted. All anchor bolts shall have single self-locking hex nuts. Tack
welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in
accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only), shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Atter the nuts have been snug tightened the anhor bot hreads shall be distorted to prevencremoval of the nuts. Distorted threads RESILIENT AND NEOPRENE PADS:
Resilient and Neoprene pads shall be in accordance with Specification Section 932 except that testin of the
JoInTs:
All welded joints are to be ground smooth. Expansion joints shall be spaced at a maximum 40'-0". Field splices similar to the expansion joint detail may be approved by the Engineer to facilitate handling, but top rail must be continu
All weld
All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel)
ANSI/AWS D1.1 (current edition). Weld metal shall be E60xX or E70xX. Nondestructive testing of weld is not require
COATINGS:
The steel railing shall be hot-dip galvanized after fabrication in accordance with Section 962 of the Specifications mill finish unless otherwise noted in the Contract Documents. All nuts, bolts and washers shall be hot-dip galvanized in accordance with Specification Section 962.
SHOP DRAWINGS:
Complete details addressing project specific geometry (line \& grade) showing post and expansion joint locations, post and panel type, anchor bolt installation "Case" or lengths, must be submitted by the accordance with the Specifications.
accordan
PAYMENT:
Railing shall be paid for per linear foot (Item No. 515-2-abb). Payment will be plan quantity measured as the length along the center line of the top rail, and includes rails, posts, pickets, pane/s, rail splice assemb/y, required to complete installation of the railing.
(Showing Inside Face of Railing with Type "A" Posts)

Note. Non-continuous corners are permitted
when handrails are not required.
EXPANDED ELEVATION AT CORNERS = DETAIL FOR NON-CONTINUOUS = RAILING AT CORNERS
NOTES:

* Keyed construction joints in Index No. 6011 Gravity
Wall are not considered to be expansion joints.
 continuous at landings between runs)

$30^{\prime}-0^{\prime \prime}$ Max. for Slopes $>6.25 \%$
$40^{\prime}-0^{\prime \prime}$ Max.


## RAMP REQUIREMENTS <br> or slopes greater than $5 \%$. Max. ramp slope $=8.33 \%$ Max. ramp cross-slope $=2.0 \%$

F'-0, L" Min.
LANDING REQUIREMENTS Max. landing slope $=2 \%$ Max. landing cross-slope $=2 \%$

RAILINGS ON GRADES STEEPER THAN 5\% (Type 1 - Picket Railing Shown, Other Types Similar)

| $\begin{gathered} \hline \text { LAST } \\ \text { REVISION } \\ 01 / 01 / 11 \end{gathered}$ |  |  | FDOT DESIGN STANDARDS <br> FY 2012/2013 | STEEL PEDESTRIAN/BICYCLE RAILING | $\begin{gathered} \text { INDEX } \\ \text { NO. } \\ 852 \end{gathered}$ | $\begin{gathered} \text { SHEET } \\ \text { NO. } \\ 2 \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |





TYPE 1 - PICKET INFILL PANEL
PICKET NOTES
on a PIC for sta da
Picket Spacing of $61 / 2 / 1$ centers is based on a $3 / 1 / 1$ NPS for standard applications,
When shown in the Contract Plans a $4 / 2 / 2$ picket spacing may be required. If an When shown in the Contract Plans a 41/2" picket spacing may be required.
alternate design is used, maintain a maximum clear opening of $5 \% s^{\prime \prime}$ for
standard installations and $378^{\prime \prime}$ for special conditions.
A)



TYPE 2-CHAIN-LINK (Continuous Infill Panel)
NOTES

1. See Plans for Infill Panel option required.

| LAST REVISION <br> 01/01/11 |  |
| :---: | :---: |

FDOT DESIGN STANDARDS FY 2012/2013



typical section on concrete sidewalk typical section on retaining wall (Case I)
(Case II)

(Cast-In-Place Anchor Bolts shown
Adhesive Anchors similar)

| $\begin{gathered} \text { LAST } \\ \text { REVISION } \end{gathered}$ | (1) DESCRIPTION: |
| :---: | :---: |


| INDEX | SHEET |
| :---: | :---: |
| NO. | NO. |
| 852 | 8 |

