

3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 54" Height Similar)

TABLE 1 - RAILING MEMBERS							
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS				
Post "A"	HSS2x2x ¹ / ₈	2.00" x 2.00"	0.125"				
Post "B"	HSS2x2x¾ ₁₆	2.00" x 2.00"	0.188"				
Ton Dail	2½" NPS (Sch. 10)	2.875"	0.120"				
Top Rail	HSS3.000x0.120	3.000"	0.120"				
End Hoons	2½" NPS (Sch. 10)	2.875"	0.120"				
End Hoops	HSS3.000x0.120	3.000"	0.120"				
Top Rail Joint/Splice Sleeves	HSS2.500x0.125	2.500"	0.125"				
Intermediate & Bottom Rail	HSS2x2x³∕ ₁₆	2.00" x 2.00"	0.188" (1)				
Int. & Bottom Rail Post Connection Sleeve	HSS1.500x0.125	1.500"	0.125" (1)				
Handrail Joint/Splice Sleeves	1" NPS (Sch. 40)	1.315"	0.133"				
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"				
Handrail Support Bar	¾" Ø Round Bar	0.750"	N/A				
Pickets (Type 1 Infill Panel)	¾" Ø Round Bar	0.750"	N/A				
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies				

TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be $1\frac{1}{4}$ " NPS (Sch. 40).

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DESIGN LOADS, GEOMETRY AND APPLICABILITY:

See the Instructions for Design Standards for the design loads, geometry and applicability requirements.

Adequate foundation support shall be provided for anchorage and stability against overturning. See Index No. 851 for special requirements and modifications for use on bridges. The railing shown on these drawings requires a handrail for ramps steeper than a 5% grade to conform with the requirements of the Americans with Disabilities

NOTES

RAILS, PANELS AND POSTS:

Pipe Rails and Pickets shall be in accordance with ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) or ASTM A36 for bars. Structural Tube shall be in accordance with ASTM A500 Grade A, B, C or D, or ASTM A501. Perforated panels (Type 5), U-Channels and filler plates shall be ASTM A36 or A1011 (Grade 36). Posts and End Rails shall be fabricated and installed plumb, \pm 1" tolerance when measured at 3'-6" above the foundation. Pickets and vertical panel elements shall be fabricated parallel to the posts, except that Type 2, 3 & 5 panel infills may be fabricated parallel to the longitudinal grade. Corners and changes in tangential longitudinal alignment shall be made continuous with a 9" bend radius or terminate at adjoining sections with mitered end sections when handrails are not required. For changes in tangential longitudinal alignment greater than 45°, posts shall be positioned at a maximum distance of 2'-0" each side of the corner and shall not be located at the corner apex. For curved longitudinal alignments the top and bottom rails and handrails shall be shop bent to match the alignment radius.

BASE PLATES AND RAIL CAPS:

Base Plates and Rail Cap Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36. SHIM PLATES:

Shim Plates shall be steel in accordance with ASTM B209. Alloy 6061 or 6063. Shim plates shall be used for foundation height adjustments greater than $\frac{1}{4}$ " and localized irregularities greater than $\frac{1}{8}$ ". Field trim shim plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of trimmed flat shim plates shown. Stacked shim plates must be bonded together with adhesive bonding material and limited to a maximum total thickness of $\frac{1}{2}$ ", unless longer anchor bolts are provided for the exposed thread

ANCHOR BOLTS:

Anchor bolts shall be in accordance with ASTM F1554 Grade 36. Headless anchor bolts for Adhesive Anchors shall be threaded full length. Cutting of reinforcing steel is permitted for drilled hole installation. Expansion Anchors are not permitted. All anchor bolts shall have single self-locking hex nuts. Tack welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only), shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and tack welds shall be coated with a galvanizing compound in accordance with the Specifications.

RESILIENT AND NEOPRENE PADS:

Resilient and Neoprene pads shall be in accordance with Specification Section 932 except that testing of the finished pads shall not be required. Neoprene pads shall be durometer hardness 60 to 80. JOINTS:

All welded joints are to be ground smooth. Expansion joints shall be spaced at a maximum 40'-0". Field splices similar to the expansion joint detail may be approved by the Engineer to facilitate handling, but top rail must be continuous across a minimum of two posts. Only use the Continuity Field Splice (Detail "E") to make the top rail continuous for unforeseen field adjustments.

WELDING:

All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal shall be E60XX or E70XX. Nondestructive testing of welds is not required.

COATINGS:

The steel railing shall be hot-dip galvanized after fabrication in accordance with Section 962 of the Specifications mill finish unless otherwise noted in the Contract Documents. All nuts, bolts and washers shall be hot-dip galvanized in accordance with Section 962 of the Specifications.

SHOP DRAWINGS:

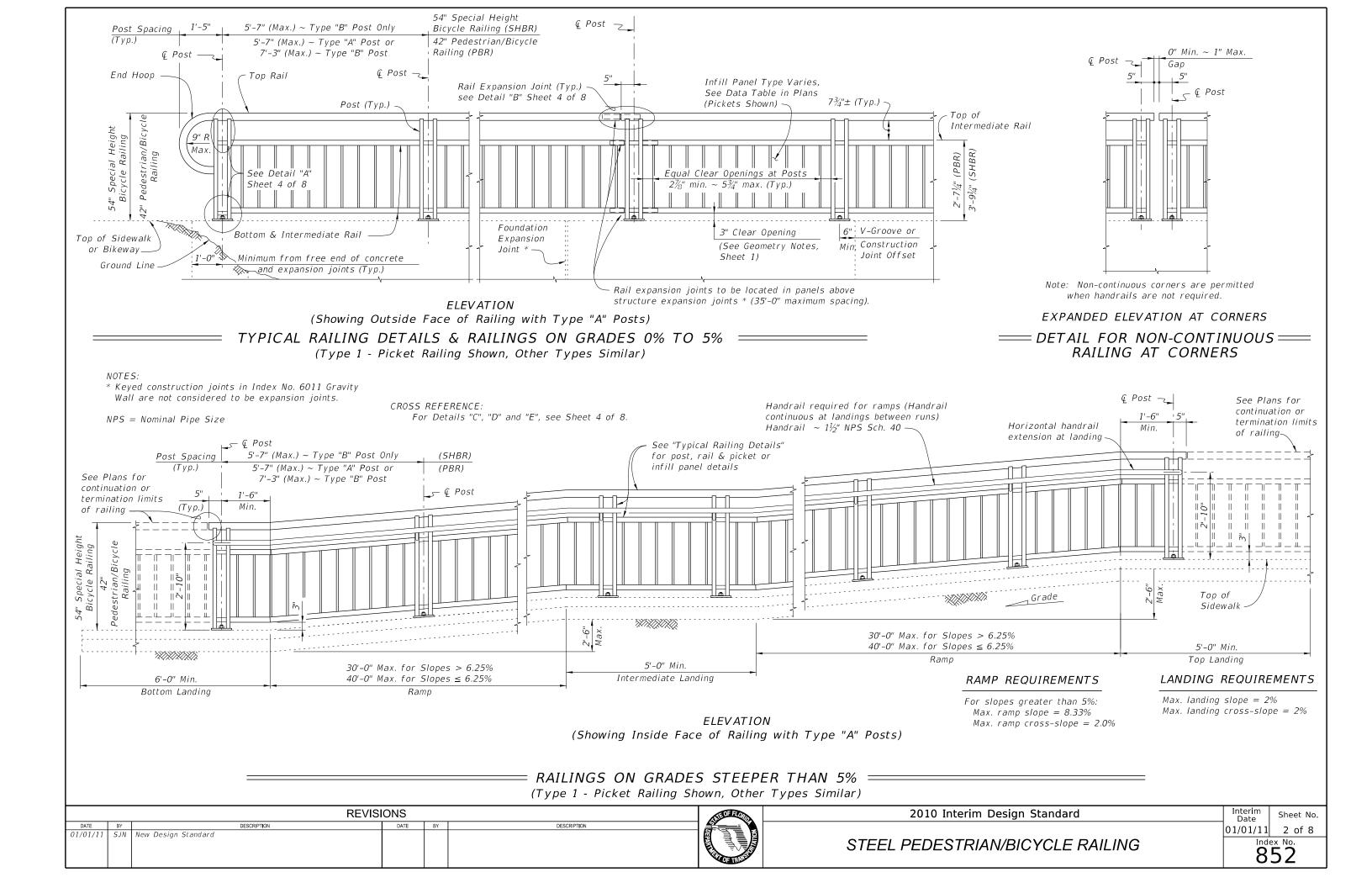
Complete details addressing project specific geometry (line & grade) showing post and expansion joint locations, post and panel type, anchor bolt installation "Case" or lengths, must be submitted by the Contractor for the Engineer's approval prior to fabrication of the railing. Shop drawings shall be in accordance with the Specifications.

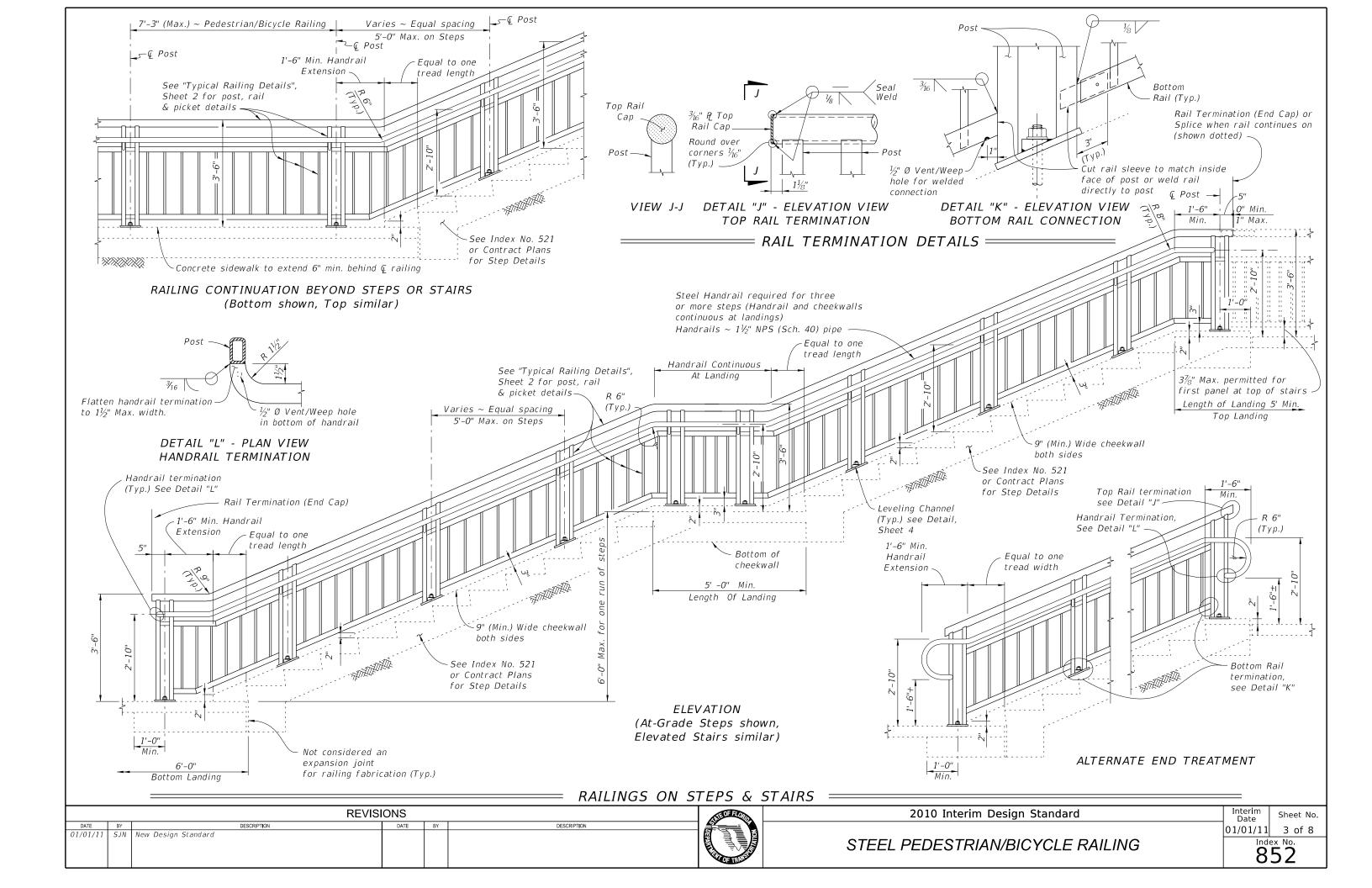
Railing shall be paid for per linear foot (Item No. 515-2-abb). Payment will be plan quantity measured as the length along the center line of the top rail, and includes rails, posts, pickets, panels, rail splice assembly, base plates, anchor bolts, nuts, washers, resilient or neoprene pads and all incidental materials and labor required to complete installation of the railing.

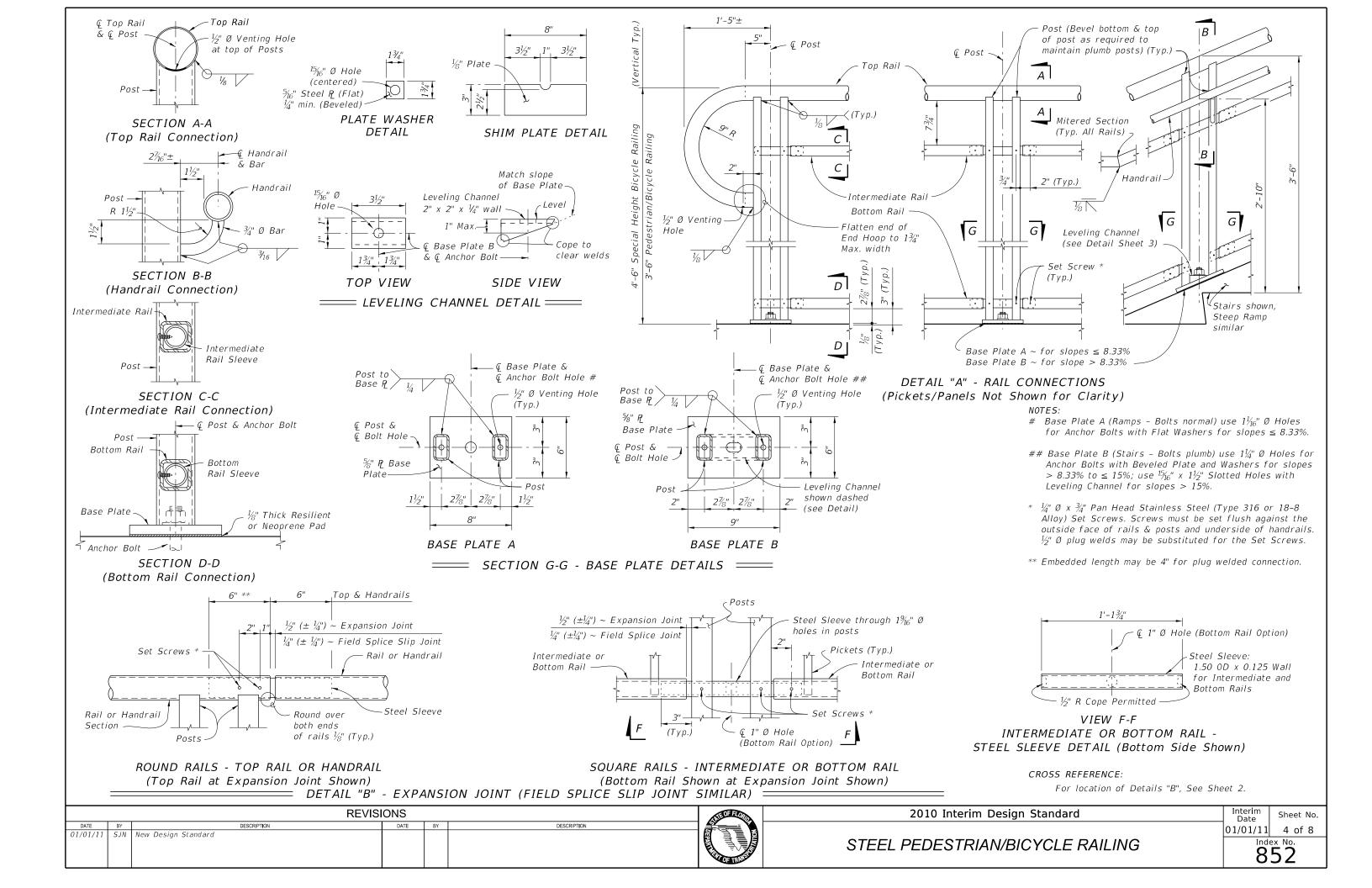
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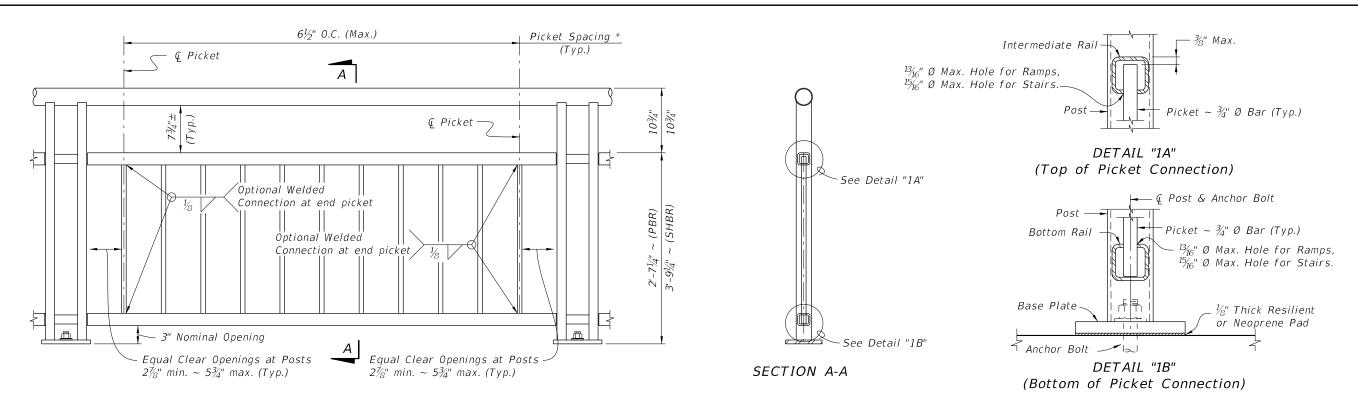
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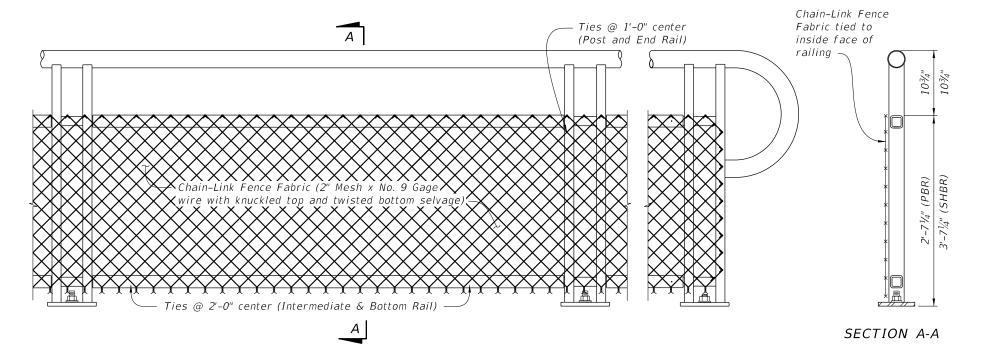




TYPE 1 - PICKET INFILL PANEL

PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " NPS for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. If an alternate design is used, maintain a maximum clear opening of $5\frac{7}{8}$ " for standard installations and $3\frac{7}{8}$ " for special conditions.



TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES

1. See Plans for Infill Panel option required.

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS						
COMPONENT	ASTM	COMPONENT INFORMATION				
Chain-Link Fence Fabric (2" mesh with	A 392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating				
twisted bottom and knuckled top selvage)	A 491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)				
	F 668	Polyvinyl Chloride (PVC) Coated Steel - No. 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.				
Tie Wires	F 626	Zinc-Coated Steel Wire - No. 9 gage with coating to match Chain-Link Fence Fabric.				
Tension Bars	F 626	$\frac{3}{16}$ " (min. thickness) x $\frac{3}{4}$ " (min. width) x 2'-3' (min. height) Steel Bars				
Miscellaneous Fence Components	F 626	Zinc-Coated Steel				

CHAIN-LINK PANEL NOTE:

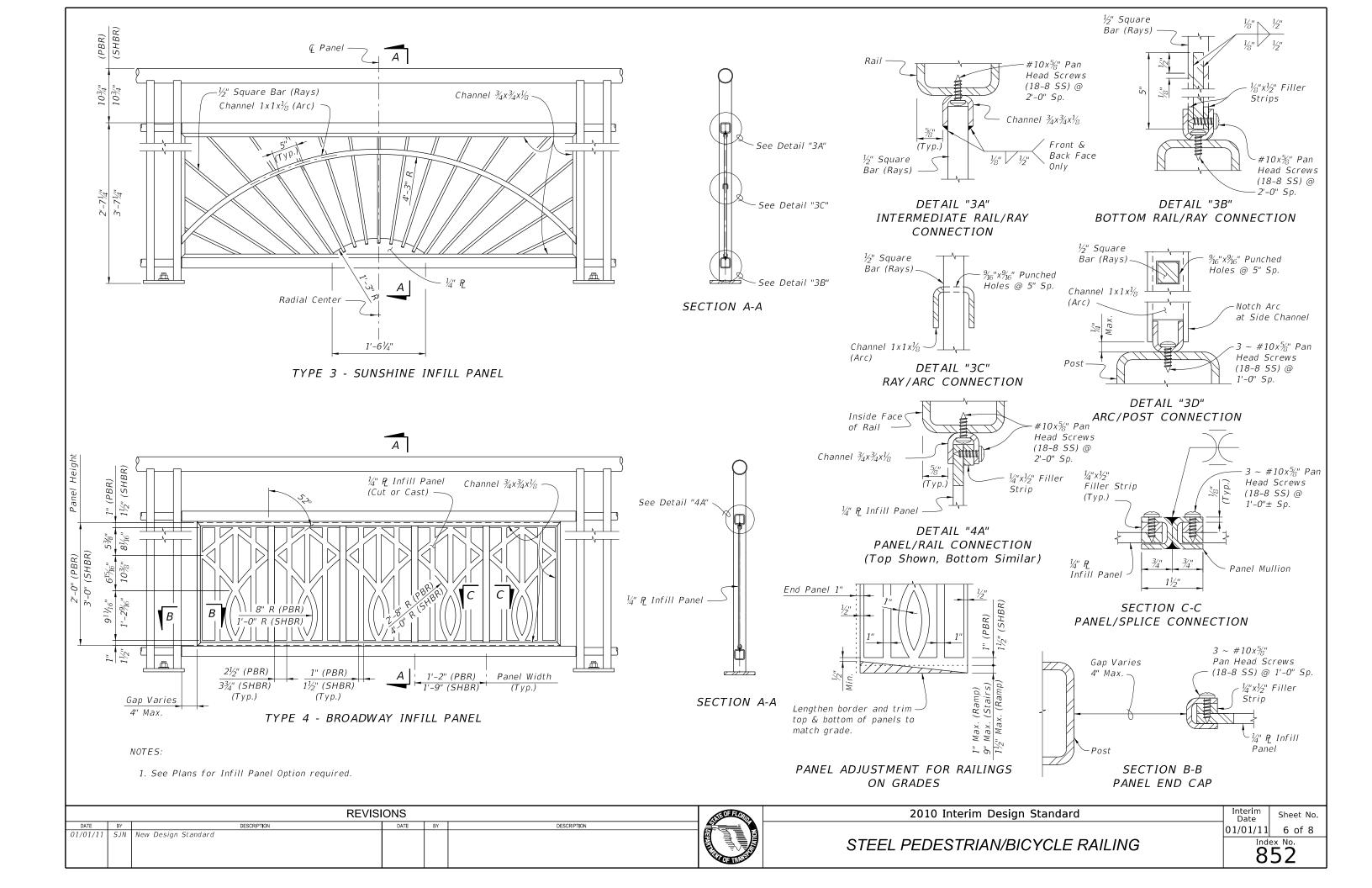
Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

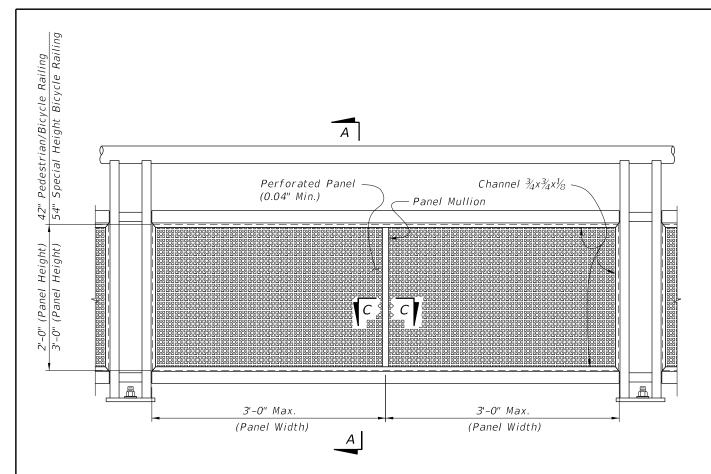
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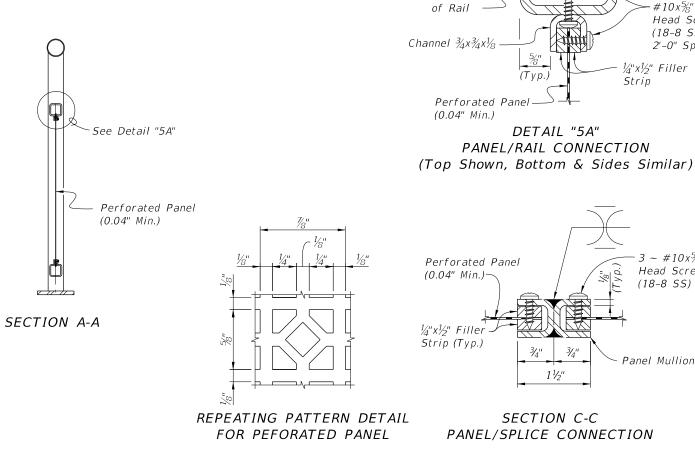
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TYPE 5 - PERFORATED INFILL PANEL



Inside Face

NOTES:

1. See Plans for Infill Panel Type required.

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-#10x5%" Pan Head Screws (18-8 SS) @ 2'-0" Sp.

3 ~ #10x5/″ Pan

Panel Mullion

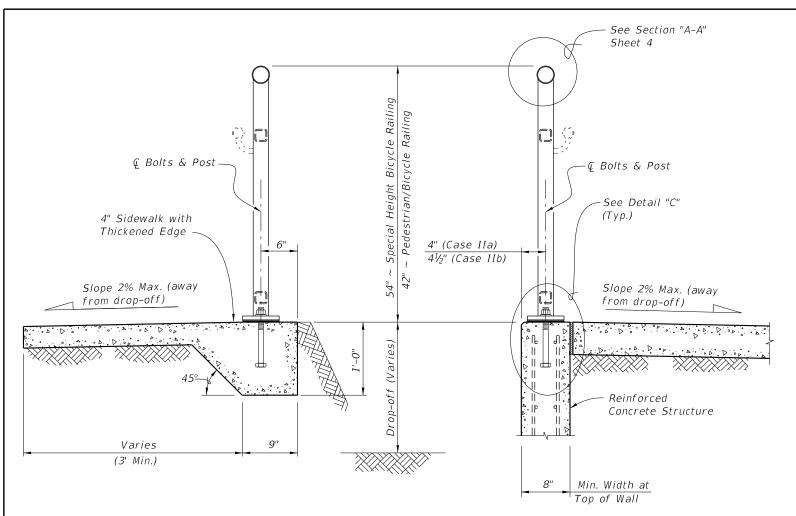
Head Screws (18-8 SS) @ 1'-0"± Sp.

¼"x½" Filler

Strip

STEEL PEDESTRIAN/BICYCLE RAILING

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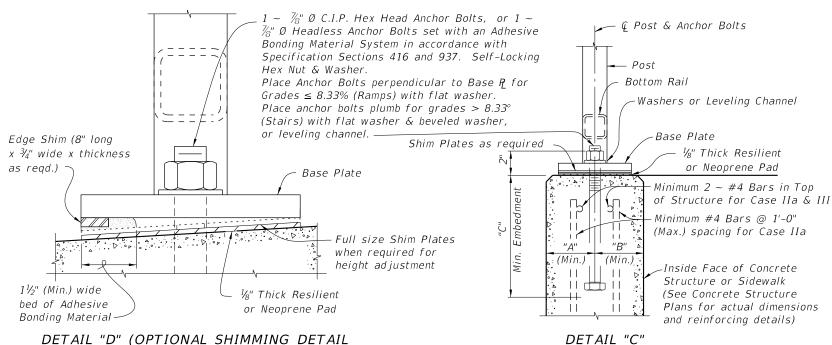


TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

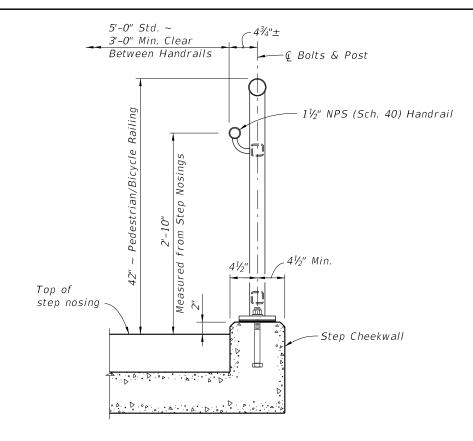
FOR CROSS SLOPE CORRECTION)

(Used in lieu of Beveled Shim Plates)

TYPICAL SECTION ON RETAINING WALL (Case II)



DETAIL "C" (Cast-In-Place Anchor Bolts shown, Adhesive Anchors similar)



TYPICAL SECTION ON STEPS & STAIRS (Case III)

	ANCHOR BOLT TABLE								
CASE	STRUCTURE		DIMENSIONS	ANCHOR	ANGUOD				
CASE	TYPE	"A" Edge Dist.	"B" Edge Dist.	"C" Embedment	C.I.P Hex Head Bolt	Adhesive Anchor	ANCHOR SIZE		
I	Unreinforced Concrete	6"	1'-2"	9"	10½"	11"	½" Ø		
IIa	Reinforced Concrete	4"	4"	9"	10½"	11"	½" Ø		
IIb	Gravity Wall Index No. 6011	4½"	3½" @ top	1'-0" *	1'-1 ¹ / ₂ "	1'-2"	%" Ø		
III	Step Cheekwall	4½"	41/2"	9"	10½"	11"	%" Ø		

^{*} Embedment length "C" may be reduced to 9" for the 42" height railings for Case IIb, when the post spacing does not exceed 5'-0".

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