

# DESIGN STANDARDS

FOR DESIGN, CONSTRUCTION, MAINTENANCE AND UTILITY OPERATIONS ON THE STATE HIGHWAY SYSTEM

2010

TOPIC NO. 625-010-003

Approved For Use On Federal Aid Projects

For Martin Knopp, Division Administrator

State of Florida, Department Of Transportation Roadway Design Office Mail Station 32 605 Suwannee Street Tallahassee, Florida 32399-0450

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# CERTIFICATION STATEMENT

I hereby certify that this Design Standard Book was compiled under my responsible charge from designs prepared, examined, adopted and implemented by the Florida Department of Transportation in accordance with established procedures, and as approved by the Federal Highway Administration.

As To Structures Design Standards Nos. 199 289-292 302 (Sheets 2-4) 306 403 411 414 420-425	As To Roadway Design Standards Nos.  001-106 200-288 293,295 300-301 302 (Sheet 1) 303-305 307-310 400-402	As To Planning Design Standard No. 17900	Manager, Traffic Data Section Transportation Statistics Office Richard L. Reel, Jr. P.E. No. 22400		
470-490 $501,505$ $521$ $530$ $810-880$ $5100-5301$ $11200-11860$ $13417$ $17502$ (Sheets $3-7$ ) $17515$ $17723,17725$ $17743,17745$ $17749$ $20110-21930$	410 $412$ $415,417$ $430$ $461$ $500$ $506-520$ $525-527$ $532-540$ $546,560$ $600-670$ $700$ $800-803$ $17302-17501$ $17502$ (Sheets 1,2) $17504,17505$	As To ITS Design Standard Nos. 18100-18305	Deputy State Traffic Operations Engineer Mark C. Wilson P.E. No. 46780  Sig: Date:		
State Structures Design Engineer Robert V. Robertson, Jr. P.E. No. 36160 Sig: Date:	17600,17721 177727-17736 17748 17764-17890 State Roadway Design Engineer David C. D'Hagan P.E. No. 33713	As To Landscape Architecture Design Standard No. 544	State Transportation Landscape Architect Jeff H. Caster LA0001592  Sig: Date:		

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		Design Standards 2010								
Index Number	Sheet Number	Description	Index Number	Sheet Number	Description					
001	1 thru 3	Added the following standard abbreviations:	233	1 thru 2	Index was expanded due to font size change.					
		Base Line, Base Line Control F Flow Line	234	1 thru 2	Index was expanded due to font size change.					
		GRI Geosynthetic Research Institute HDPE High Density Polyethylene NPS Nominal Pipe Size		2 of 2	Under Pavement & Sodding detail changed "1/2" Exp. Joint" to "1/2" Preformed Joint Filler".					
		Deleted the following standard abbreviations:  Bbl Barrel	235	1 of 2	"GENERAL NOTES", Note 3, deleted "Alternate B" replaced with "Index 200", Note 8 changed "Specification Section 962" to "Specification Section 975".					
		FRCP Fiber Reinforced Concrete Pipe FRP Fiber Reinforced Pipe FS Far Side	245	1 of 1	"GENERAL NOTES" Note 2, delete and replace with the following: "Concrete shall be Class I (Structural), except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants					
002	2 of 3	Deleted Hand Drafting Symbols			meeting the requirements of Section 449 of the Specifications. Box shall be reinforced with No. 3 bars					
102	2 of 3	NOTES FOR SYNTHETIC BALES OR BALE TYPE BARRIERS, Note 2, deleted the text "trenched 3" to 4" and" from the first sentence.	250	1 of 2	(Grade 60) on 8" centers both ways, sides and bottom.  "GENERAL NOTES" Note 5, deleted and replaced with the following: "Concrete shall be Class I					
104	2 of 2	RURAL DIVIDED detail, changed "5" Shoulder Pavement" to "4" Shoulder Pavement".			(Structural), except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications."					
105	1 of 1	TREATMENT I, Criteria for using Treatment I, replaced text of the last bullet with the following: "resurfacing build-up is less than 3" ".	251	1 of 2	"GENERAL NOTES" Note 4, deleted and replaced with the following: "Concrete shall be Class II, except					
200	1 of 5	TOP SLAB REINFORCING STEEL DIAGRAM (ALTERNATE B) to the notes "2 Additional Bars A @ 5"  D.C." and "2 Additional Bars B @ 5" Max. D.C. Each Side Of Opening", added "(Minimum #4 Bars)".			ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications."					
	2 of 5	Note 9, Delete second sentence and substitute, "Additional bars used to restrain hole formers for precast structures with grouted pipe connections, may be left flush with the hole surface."	252	1 of 2	"GENERAL NOTES" Note 4, deleted and replaced with the following: "Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications."					
	4 of 5	SLAB AND WALL DESIGN TABLE NOTES, added the following to the end of Note 10: "See Index No. 201, Sheet 4 for allowable bar spacing adjustments when larger areas of reinforcing are substituted."	253	1 of 2	"GENERAL NOTES" Note 4, deleted and replaced with the following: "Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications."					
201	4 of 5	"Revised title of notes to ""NDTES FOR PRECAST OPTIONS AND EQUIVALENT REINFORCEMENT SUBSTITUTION"" and added the following to Note 4, ""When an increased area of reinforcing is provided, then the maximum bar spacing may be increased by the squared ratio of increased steel area, but not to exceed 12 inches:	255	1 of 2	"GENERAL NDTES" Note 4, deleted and replaced with the following: "Concrete shall be Class II, except ASTM C478 (4000 psi) concrete may be substituted for precast items manufactured in plants meeting the requirements of Section 449 of the Specifications."					
		Max. Bar Spacing Provided < Max. Bar Spacing Required x (Steel Area Provided/Min. Steel Area Required) <sup>2</sup> "	260	1 of 1	"GENERAL NOTES" Note 3 changed "Specification Section 962" to "Specification Section 975".					
205	1 of 6	Changed maximum size of allowed PVC pipe to 36".	261	1 of 3	"GENERAL NOTES" Note 4 changed "Specification Section 962" to "Specification Section 975".					
	2 of 6	ROUND PIPE DIMENSIONS, deleted the column, "Wall Thickness (In.) Class III" and subcolumn	264	1 thru 2	Index was expanded due to font size change. General note 3 changed.					
		"NRCHP" and heading "SRCP". Also deleted the ** note at the bottom of the table.	270	1 of 1	"GENERAL NOTES" Note 2 changed "Specification Section 941–1.5" to "Specification Section 449". Changed Note 3.					
	3 of 6	NOTES: deleted note 4; table "PIPE ARCH: SPIRAL RIB: $\frac{3}{4}$ " x $\frac{3}{4}$ " x $\frac{7}{2}$ " RIB SPACING" deleted references to note 4; table "RDUND PIPE – SPIRAL RIB", "Maximum Height of Fill (Ft.)", "Sheet Thickness In Inches (Gage)", "0.138 (10)" added measurements.	272	6 of 6	Reordered "GENERAL NOTES" and changed "Class I concrete" to "Class NS concrete".					
210	1 of 1	Delete General Note 4, and substitute the following: "For precast units the rear wall and apron may	273	1 thru 7	Index was expanded due to font size change.					
		be precast as a separate piece from the top slab. Provide a minimum of $7 \sim \#4$ dowels in accordance with Index No. 201 "OPTIONAL CONSTRUCTION JOINTS".		7 of 7	"GENERAL NOTES", Note 8, deleted "Class I concrete" and substituted "Class NS concrete".					
211	1 thru 5	Revised index completely 3 sheets added, Reinforcing configuration and C.I.P. details revised; precast and WWR details added. Changed Note 4 to allow 4'-0'' round risers.	280	1 thru 3	Index was expanded due to font size change.					
213	1 of 1	In PLAN view changed "1/2" Exp. Joint (Typ)" to "1/2" Preformed Joint Filler (Typ)".		1 of 3	"DISSIMILAR TYPES CONCRETE JACKET FOR CONNECTING DISSIMILAR TYPES OF PIPE AND CONCRETE PIPES WITH DISSIMILAR JOINTS" detail, added the note, "Alternate connection must be approved by the State Drainage Engineer."					
218	2 of 2	"STEEL GRATE", "TOP VIEW", for the overall dimension on the left side of the grate, inserted " $44^{1}/_{4}$ " ". For the small dimension at the upper left corner of the grate, inserted " $3^{1}/_{2}$ " ".	282	1 thru 3	Index was expanded due to font size change.					
219	1 of 2	In PLAN view and Section HH changed "Expansion Joint (Typ)" and "Expansion Material Joint" to "1/2" Preformed Joint Filler (Typ)".		1 of 3	"FRONT ELEVATION" and "SECTION AA" details changed "1/2" Exp. Matl. " to "1/2" Preformed Joint Filler".					
220	1 of 3	"GUTTER INLET TYPE S", "SECTION BB", Changed the vertical dimension between the top of the inlet and the grate elevation from " $5\frac{1}{2}$ " to " $4\frac{1}{2}$ " ".	284	2 of 3 1 of 1	"PLAN" and "SECTION AA" details changed "1/2" Exp. Matl. " to "1/2" Preformed Joint Filler".  Deleted note "1" and substituted the following: "1. Spillway to be paid for as Shoulder Gutter, LF."  Deleted note "2", and substituted the following: "2. If spillway empties into an unpaved ditch the					
		"SECTION AA", at the top right corner, for precast thickness changed " 6" " to " 3" " (same as left side).	287	1 thru 4	detail should be modified as necessary."  Sheet 3 is new. Renumbered other sheets.					
		"SECTION BB", at the top, changed "3'-11" Precast" to " 4'-3" Precast". "PLAN", at the top,		1 of 4	Changed all 3 occurrences of "Class I concrete" to "Class NS concrete".					
		changed " 3'-11" Precast to " 4'-3" Precast".	288	1 of 1	New Index added "DEEP WELL INJECTION BOX".					
230	1 of 2	In "PLAN" view changed "1/2" Exp. Joint (typ)" to "1/2" Preformed Joint Filler (Typ)". Section E-E, Changed 4Z15.9 shape to built up section (3.5 x 3 x $\frac{1}{2}$ L + $\frac{1}{2}$ x 3 Bar) for grating.	289	6 of 7	Changed "FLARED ENDWALL" to "FLARED WINGWALL" and "STRAIGHT ENDWALL" to "STRAIGHT WINGWALL".					
231	1 of 3	"DITCH BOTTOM INLET TYPE B", "SECTION BB", upper left side, deleted the dimension "2'-6"	291	1 of 5	Changed "Class I Concrete" to "Class NS".					
272	1 +6~ 7	(Min.)" and replaced with "1'-10" (Min.)".	200	5 of 5	Changed "Bond Beam" to "Link Slab", and "Class I Concrete" to "Class NS".					
232	1 thru 7	Index was expanded due to font size change.	292	2 of 14	"GENERAL NOTES" note 1, changed AASHTO LRFD Bridge Specifications, to "4th Edition"; added note 10.					

Index	Sheet	T	Sheet	T	
Number	Number	Description	Index Number	Number	Description
295	1 of 1	"GENERAL NOTES" Note 2 changed "Specification Section 962" to "Specification Section 975".	421	1 of 3	Changed REFLECTIVE RAILING MARKERS note, "Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing along the centerline at the spacing shown in the specific results of the specific results."
300	1 thru 2	Index was expanded due to change in font.			in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."
304	6 of 6	Added alternate location of detectable warnings on linear ramps. Added note "On curb ramps, landings and flush transitions perpendicular to the curb line: Rows of domes shall be aligned with the centerline of the ramp. (See Pictorial View A)" at top of sheet. Added Rail Road Crossing PLAN view.	422	1 of 3	Added the following to the NAME, DATE AND BRIDGE NUMBER note: "The Name shall be as shown in the General Notes in the Structures Plans."; Changed REFLECTIVE RAILING MARKERS note.
305	1 & 4 of 4	Deleted bar spacing table and revised notes (Sheet 1); Changed width of outside lanes (Sheet 4).			Changed REFLECTIVE RAILING MARKERS note, "Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the
307	2 of 3	"UTILITY CONFLICT PIPES THRU STORM SEWER STRUCTURES" changed to "UTILITY CONFLICT PIPES THRU STORM DRAIN STRUCTURES"			near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."
310	1 of 2	"SIDEWALK WITH EDGE BEAM FOR SURFACE MOUNTED RAILINGS", "Clear Width", deleted "3' Min." and substituted "4' Min. *".	423	1 of 3	Added the following to the NAME, DATE AND BRIDGE NUMBER note: "The Name shall be as shown in the General Notes in the Structures Plans."; Bicycle Railing to "Special Height Bicycle Railing" and Post "B" to Post "B1".
		"NOTES FOR CONCRETE SIDEWALK ON CURBED ROADWAYS", deleted "Note 1", and substituted the following: "1. Sidewalks shall be constructed in accordance with Section 522 of the FDOT Standard Specifications. Public sidewalk curb ramps shall include detectable warnings and be constructed in accordance with Index No. 304. Detectable warnings are not required where sidewalks intersect urban flared turnouts."			"TRAFFIC RAILING-(32" VERTICAL SHAPE)", deleted the "REFLECTIVE RAILING MARKERS" note and substituted the following: "Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."
		"Note 3" , deleted.		2 of 3	Changed Bicycle Railing to "Special Height Bicycle Railing" and Post "B" to Post "B1".
	2 of 2	"NOTES FOR CONCRETE SIDEWALKS ON UNCURBED ROADWAYS", Changed Note 2 to "Provide detectable warnings that extend the full width of the sidewalk and 24" deep from the edge of pavement where sidewalks adjoin the following vehicular ways:		3 of 3	Changed 83 degrees to 93 degrees in CONVENTIONAL REINFORCING STEEL BENDING DIAGRAM Cross-slope table.
		side roads and streets driveways with signalized entrances driveways with entrance volumes greater than 600 vpd	424	1 of 7	Added the following to the NAME, DATE AND BRIDGE NUMBER note: "The Name shall be as shown in the General Notes in the Structures Plans."
400	1.11.00	driveways with entrance speeds of 25 mph or greater right in - right out composite driveways.			"TRAFFIC RAILING - (CORRAL SHAPE)", deleted the "REFLECTIVE RAILING MARKERS" note and substituted the following: "Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in
400	1 thru 26	Index expanded by one sheet due to font size change and added new sheet 2, "APPROACH END ANCHORAGE DETAILS", Index renumbered.			the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."
	1 of 26 2 of 26	"GENERAL NOTES" Note 17 changed "Specification Section 971" to "Specification Section 975".  New sheet added showing limits of pay for guardrail, details of shoulder treatment and miscellaneous	425	1 of 3	Added the following to the NAME, DATE AND BRIDGE NUMBER note: "The Name shall be as shown in the General Notes in the Structures Plans."
	7 (00	asphalt for guardrail approach end treatments.			"TRAFFIC RAILING - (42" F SHAPE)", added the following note: "REFLECTIVE RAILING MARKERS:
		Corrected spelling of guardrail in last paragraph.			Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector
	15 of 26	"LOCATIONS ON FRONT SLOPES", deleted the details for guardrail on slope and rubrail termination and the chart for lateral placement on slopes. (See sheet 26)			color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."
	16 of 26	Deleted "REFLECTORS- DETAIL M" (See sheet 17)	470	1 ( 7	
	26 of 26	Added "GUARDRAIL ON SLOPES", details for guardrail on slope and rubrail termination and the chart for lateral placement on slopes.	470	1 of 3	Added Field testing proof loads to the ADHESIVE BONDED ANCHORS AND DOWELS note; "TRAFFIC RAILING-(THRIE BEAM RETROFIT) GENERAL NOTES & DETAILS", deleted the "BRIDGE NAME PLATE" note and substituted the following: "If a portion of the existing Traffic Railing is to be removed
410	1 thru 25	Index completely revised and reorganized.			that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that
411	2 of 10 4 of 10	Changed tangent offsets In Detail 'A' to ''2.49'-Design Speed ≤45 mph; 1.76' - Design Speed ≥50 mph''. Changed tangent offsets In Detail 'B' to ''2.49'-Design Speed ≤45 mph; 1.76' - Design Speed ≥50 mph''.			has been removed or obscured, with 3"tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of
414	1 of 15	Updated Specification reference Section 971 to 975; Added steeloption to ALTERNATE DESIGN note.			the approaching travellane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers."
	5 of 15	Added PTFE tape option to anchor bolt details.			Added the following note: "NEOPRENE PADS: Neoprene pads must be plain pads with a durometer
415	4 of 10	"NOTES FOR WALL END SHIELDING", Note 1, changed the second sentence to: "Except where the plans designate a particular type crash cushion for a specific location, the contractor has the option to construct any of the redirective crash cushions listed on the Qualified Products List, subject to			hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required."
		the uses and limitations described on their respective drawings."		3 of 3	Changed offset of $\frac{7}{8}$ " dia. anchor bolts to $2\frac{3}{4}$ " from back edge of base plate in SECTION B-B.
		"ANCHOR PLATE BOLTS", upper note, changed "?" to "3/4"".	471	2 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad".
420	1 of 3	Added the following to the NAME, DATE AND BRIDGE NUMBER note: "The Name shall be as shown in the General Notes in the Structures Plans."; Changed REFLECTIVE RAILING MARKERS note.	472	2 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad".
		Changed REFLECTIVE RAILING MARKERS note, "Reflective Railing Markers shall meet Specification	473	2 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad".
		Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing."	474	2 of 4 4 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad". "SECTION C-C", changed "Resilient Pad" to "Neoprene Pad".

Index Number	Sheet Number	Description	Index Number	Sheet Number	Description
475	2 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad".	600	3 of 13	LANE WIDTHS, in the second sentence, change the word "expected" to "excepted".
476	2 of 4	"SECTION A-A" and "SECTION B-B", changed "Resilient Pad" to "Neoprene Pad".		5 of 13	Changed note under "SIGN COVERING AND INTERMITTENT WORK STOPPAGE SIGNING"; added
480	1 of 2	"TRAFFIC RAILING-(VERTICAL FACE RETROFIT) GENERAL NOTES & DETAILS", added the following to the "ADHESIVE-BONDED ANCHORS AND DOWELS" note, "The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0" embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5" min. embedment)." Added NEOPRENE PADS note.  Also deleted the "REFLECTIVE RAILING MARKERS" note and substituted the following: "Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2"		6 of 13	information for the use of the new "PRDJECT INFORMATION SIGN".  GENERAL NOTES, deleted note 1, substituted the following: "1. All signs shall be post mounted when work operations exceed one day except for: a) Road closure signs mounted in accordance with the vendor drawing for the Type III Barricade shown on the QPL. b) Pedestrian advanced warning or regulatory signs mounted on sign supports shown on the QPL."
		from the face on the traffic side at the spacing shown in the table below. Reflector color (white or yellow) shall match the color of the near edgeline."			"2 POST SIGN SUPPORT MOUNTING DETAILS", updated text to include a tolerance between sign supports. Insert "+/- 3" " after "1'-6" "and insert "+/- 6" "after "2'-6" ".  POST AND FOUNDATION TABLE FOR WORK ZONE SIGNS, expanded Note 2 by adding: "unless otherwise
	2 of 2	CONVENTIONAL REINFORCING STEEL BENDING DIAGRAM, added Bars 5E, 5F and 4G for Index No. 484			specified in the vendor drawing on the QPL."
484	1-10 of 10	New Index added TRAFFIC RAILING (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH			POST MOUNTED SIGN NOTES, added new notes 1 and 12.
500	2 of 2	"HALF SECTION" detail, deleted "Storm Sewer Mains" replaced with "Storm Drain Trunk Lines"		7 of 13	Added new sheet showing Project Information Sign and renumbered index.
501	3-9 of 9	Changed the REQUIRED TEST METHOD for Burst Strength, Soil-Geosynthetic Friction, Creep Reduction Factor & Joint Overlap to ASTM D 6706.	605	1 of 1	"GENERAL NOTES", deleted the text of "Note 8" and substituted the following: "The two channelizing devices directly in front and directly at the end of the work area may be omitted provided vehicles in
	4 of 9	Updated values for COMTRAC 70.70; Deleted AMOCO 2006, 2016 & 2044; Added GEOTEX 315ST, 2x2HF, 4x4, 3x3HF, 4x4HF & 4x6 woven geogrids.			the work area have high intensity rotating, flashing, oscillating or strobe lights operating."  Added new heading "DURATION NOTE" and placed the following note under this heading:
	5 of 9	Changed Joint Strength Overlap value to 1.2 for all Marafi products.			1. RDAD WDRK AHEAD sign may be omitted if all of the following conditions are met:
	6 of 9	Deleted Application Usage 3 & 4 for SYNTEEN SF 11 & SF 12.			a) Work operations are 60 minutes or less. b) Speed is 45 mph or less.
	7 of 9	Added Fornir 20			c) No sight obstructions to vehicles approaching the work area for a distance of 600 feet. d) Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
	8 of 9	Changed Creep Resistance and Creep Reduction Factors for TENSAR BX 1120, BX 1200, BX 1220 & BX 1500			e) Volume and complexity of the roadway has been considered.
	9 of 9	Updated values for TENAX MS 220 & TENAX MS 330. Added Combigrid 30/30, Secugrid 20/20 & 30/30 extruded geogrids.	625	1 of 1	New Index added "TEMPORARY ROAD CLOSURE- 5 MINUTES OR LESS".
505	1-4 of 4	Sheet 3 is new. Renumbered other sheets.	655	1-3 of 3	New Index added "TRAFFIC PACING-LIMITED ACCESS".
515	5 of 7	In second symbolized note changed "Section 102-6" to "Section 102-8".	667	1-6 of 6	New Index added "TOLL PLAZAS".
313	6 of 7	"PAVEMENT STRUCTURE FOR TURNOUTS AND AUXILLIARY LANES TABLE 515-1", "NOTES", Note 5,	801	1 of 3	"GENERAL NOTES", Note 15 and 21, deleted "Class I" and substituted "Class NS".
		Deleted "Class I concrete" substituted "Class NS concrete".	802		Added tolerance to ground clearance; revised Notes 7a and 7b; rearranged sheets.
518		Revised width of rigid pavement outside travellane and changed location of rumble strip.		1013	"GENERAL NOTES", Note 6 and 13, deleted "Class I concrete" and substituted "Class NS concrete" for all occurrences.
520	1 of 1	"GENERAL NOTES", Note 7, Deleted "Class I Concrete (Retaining Walls)" and substituted "Class NS Concrete"	803	1 of 1	"GENERAL NOTES", Note 4, deleted both occurrences of "Class I" and substituted "Class NS".
546	1 of 6	Added detail"PLAN", "PICTORIAL" and ** note. Index sheets reordered.	810	2 of 4	Deleted "Section 971" and substituted "Section 975" in ANCHOR RODS, NUTS AND WASHERS note.
	5 of 6	Under "NOTES FOR 4-LANE DIVIDED ROADWAY", Note 1, changed reference from "Sheet 6" to	811	3 of 3	Deleted "Section 971" and substituted "Section 975" in ANCHOR RODS, NUTS AND WASHERS note.
		"Sheet 2".	812	2 of 4	Deleted "Section 971" and substituted "Section 975" in ANCHOR RODS, NUTS AND WASHERS note.
600	2 of 13	OVERHEAD WORK, deleted "OPTION 4" and substituted the following: OPTION 4 (OVERHEAD WORK MAINTAINING TRAFFIC WITH NO ENCROACHMENT BELOW THE OVERHEAD WORK AREA) Traffic shall be detoured, shifted, diverted or paced as to not encroach in the area	820	1 of 1	Changed Top Rail to "Special Height Bicycle Railing" and added new Post "B2" for 3'-6" height Pedestrian/Bicycle Railing.
		directly below the overhead work operations in accordance with the appropriate standard index drawing or detailed in the plans. This option applies to, but not limited to, the following construction activities:	821	1 of 1	Changed designation of 4'-6" tall railing to "Special Height Bicycle Railing" and added 3'-6" tall Pedestrian/Bicycle Railing.
		(a) Beam, girder and segment placement. (b) Deck form placement and removal. (c) Concrete deck placement.	822	1 of 2	Changed designation of 4'-6" tall railing to "Special Height Bicycle Railing" and "Post B" to "Post B1"; Added "Post B2" details.
		(d) Railing construction located at edge of deck. (e) Structure demolition.	850	1 of 5	Changed "Pedestrian Railing" to "Pedestrian/Bicycle Railing" and "Bicycle Railing" to "Special Height Bicycle Railing"; Added anchor bolt requirements to SHOP DRAWINGS note.
		DEFINITIONS, added the following after definition of TRAVEL WAY:  a. TravelLane: The designated widths of roadway pavement marked to carry through traffic and to separate it from opposing traffic or traffic occupying other lanes.  b. Auxiliary Lane: The designated widths of roadway pavement marked to separate speed change,		2 of 5	Added "DETAIL FOR NON-CONTINUOUS RAILING AT CORNERS" detail. Changed Pedestrain and Bicyle Railing designation; maximum ramp length for slopes less than 6.25%; and minimum clear picket opening at post to $\frac{3}{4}$ ".
		turning, passing and climbing maneuvers from through traffic.			Changed Pedestrain and Bicyle Railing designation.
		CLEAR ZONE WIDTHS FOR WORK ZONES, deleted the text "travel" in the first sentence and substituted "traffic".		4 of 5	Added requirement for set screw to be set flush against outside face of rail and 18-8 Alloy option in DETAILS "D" & "E", option to notch post in SECTION G-G, and $\frac{1}{4}$ " joint tolerance in DETAIL "D".
		Replaced chart "CLEAR ZONE WIDTHS FOR WORK ZONES".		5 of 5	Added DETAIL "F" and note (*) to ANCHOR BOLT TABLE. Changed Pedestrain and Bicyle Railing designation. Corrected height dimension on steps to top of nosing.

Index lumber	Sheet Number	Description	Index Number	Sheet Number	Description
851	1 of 2	Changed Pedestrain and Bicyle Railing designation.	5204	1 of 1	Changed "Ribbed" to "Slotted" in PLUG DETAIL.
	2 of 2	Added requirement for set screw to be set flush against outside face of rail and 18–8 Alloy option in DETAIL "B". Changed field splice joint tolerance to $\frac{1}{4}$ " in DETAIL "B".	5205	1, 3, 4 & 6 of 7	Added note in Elevation Views to 'Extend post 2" above high side wall panel when post caps are shown in the plans'.
860	1 of 5	Changed "Pedestrian Railing" to "Pedestrian/Bicycle Railing" and "Bicycle Railing" to "Special Height Bicycle Railing"; Added anchor bolt requirements to SHDP DRAWINGS note. Added filler metal ER4043		2 of 7	Added tolerance between Top of Precast Collar and Auger Cast Pile; Changed "Composite Bearing Pads" to "Fiber Reinforced Bearing Pads".
		to WELDING note.		5 of 7	Changed "Composite Bearing Pads" to "Fiber Reinforced Bearing Pads".
	2 of 5	Added "DETAIL FOR NON-CONTINUOUS RAILING AT CORNERS" detail. Changed Pedestrain and Bicyle Railing designation; maximum ramp length for slopes less than 6.25%; and minimum clear picket		7 of 7	Added "Octangonal Precast Collar" details and tolerance between Top of Precast Collar and Auger Cast Pile; Changed "Composite Bearing Pads" to "Fiber Reinforced Bearing Pads".
		opening at post to 3/4".	5206	1 of 1	Added "POST LENGTH WITH CAP" column, BARS D, P5 thru P8 to table and bar bending details for corner posts.
	3 of 5	Changed Pedestrain and Bicyle Railing designation.	5207	1 of 1	New Index added "PRECAST SOUND BARRIERS-PRECAST POST CAPITAL".
	4 of 5	Added requirement for set screw to be set flush against outside face of rail and 18–8 Alloy option in DETAILS "D" & "E"; option to notch post in SECTION G-G; $\frac{1}{4}$ " joint tolerance in DETAIL "D"; Type B (Nonwelded) connection detail in SECTION A-A. Changed Expansion Joint sleeve embedded length to 10" in DETAIL "D" and picket fillet weld size to $\frac{1}{8}$ ", handrail and top rail fillet weld size to $\frac{1}{4}$ ", and base plate fillet weld size to $\frac{3}{8}$ ".	5210	2 of 5	Changed NAME, DATE AND BRIDGE NUMBER note, and "Ribbed" to "Slotted" in NEOPRENE DIAPHRAGM PLUG DETAIL. Added REFLECTIVE RAILING MARKERS note and RELECTIVE RAILING MARKER SPACING table.
	5 of 5	Added DETAIL "F" and note (*) to ANCHOR BOLT TABLE. Changed Pedestrain and Bicyle Railing designation. Corrected height dimension on steps to top of nosing.	5211	3 of 3	Changed "Ribbed" to "Slotted" in NEOPRENE DIAPHRAGM PLUG DETAIL. Corrected Anchor Pin daimeter on FIRE HOSE ACCESS DETAIL.
861	1 of 2	Changed designation of 54" tall railing to "Special Height Bicycle Railing".	5212	2 of 2	Added note for "Full Depth Structural Asphalt" above junction slab and changed coping dimension to 6" Min.
	2 of 2	Added requirement for set screw to be set flush against outside face of rail and 18-8 Alloy option in DETAIL "B". Changed field splice joint tolerance to $\frac{1}{4}$ " and "Steel Sleeve" to "Aluminum Sleeve"	5300	3 of 19	Increased max. gap at back of precast coping and added timber blocking.
		in DETAIL "B".		6 of 19	Added note for "Full Depth Structural Asphalt" above junction slab and increased max. gap at back of precast coping.
370	1 of 5	Deleted Pedestrian and Bicycle designations from DESIGN LIVE LOADS and ALTERNATE DESIGN notes.		7 of 19	Added note for "Full Depth Structural Asphalt" above junction slab.
	2 of 5	Deleted 4'-6" Bicycle Railing option and "**" note. Changed maximum ramp length for slopes less than 6.25%.		12 & 15 of 19	Increased max. gap at back of precast coping. Corrected size of Bar 5U1 in BILL OF REINFORCING TABLE
	3 of 5	Deleted 4'-6" Bicycle Railing option.	11200	1-2 of 2	Deleted sheet 2
	4 of 5	Added requirement for set screw to be set flush against outside face of rail and 18-8 Alloy option in DETAILS "D" & "E"; and $\frac{1}{4}$ " joint tolerance in DETAIL "D". Deleted Intermediate Rails from		1 of 2	Revised and rearranged notes, sheet renumbered to 1 of 2.
	5 of 5	DETAILS "B" and "C".  Added DETAIL "F". Deleted 4'-6" Bicycle Railing option. Corrected height dimension on steps to top		2 of 2	Renumbered sheet 3 of 3 to sheet 2 of 2 revised and rearranged notes. Deleted "Class 1 (Special) Concrete" replaced with "Class 1 Concrete".
880	1 of E	of nosing.  Deleted Pedestrian and Bicycle designations from DESIGN LIVE LOADS and ALTERNATE DESIGN notes.	11300	1 of 1	Hanger table values revised; connection bolt size revised; sign depth for horizontal splice changed to 10 U-Bolt material spec (A325) added to Typical Detail of Sign & Truss Connection.
000	1 of 5 2 of 5	Deleted 4'-6" Bicycle Railing option and "**" note. Changed maximum ramp length for slopes less	11310	1 of 5	Deleted A307 bolts and Palnut (Note 4e). Changed foundation concrete (Note 7). Changed to ½" mesl (Note 9). Deleted grout pad and notes (former Notes 7c & 9). Added CSL tube note (Note 14).
	3 of 5	than 6.25%.  Deleted 4'-6" Bicycle Railing option.		2 of 5	Changed foundation standoff distance and changed drilled shaft detail. Deleted grout pad and added will screen. Added CSL tubes. Changed FC & FL reinforcing.
	4 of 5	Added requirement for set screw to be set flush against outside face of rail and 18-8 Alloy option		5 of 5	Changed bolt spacing connection details.
	5 of 5	in DETAILS "D" & "E"; and ½ joint tolerance in DETAIL "D". Deleted Intermediate Rails from DETAILS "B" and "C".  Added DETAIL "F". Deleted 4'-6" Bicycle Railing option. Corrected height dimension on steps to top	11320	1 of 5	Deleted A307 bolts and Palnut (Note 4e). Changed foundation concrete (Note 7). Changed to $\frac{1}{2}$ " mesh (Note 9). Deleted grout pad and notes (former Notes 7c & 9). Added CSL tube note (Note 14).
	5 01 5	of nosing.		2 of 5	Changed foundation standoff distance. Deleted grout pad and added wire screen.
5100	2 of 2	Changed to plastic sleeve expansion joint and "Premoulded Expansion Material" to "Preformed Joint		4 of 5	Changed bolt spacing connection details.
		Filler". Changed wall and expansion joint key.		5 of 5	Changed drilled shaft detail. Added CSL tubes.
5200	1 of 1	Post caps added to note C.1.b; Changed note K.2 to allow 8 ft height panels. Added note K.11; Changed notes H.1, H.2 and D.2; Deleted note H.3.	11860	1 of 8	Changed SINGLE COLUMN GROUND SIGN NOTES, Note 11, and GUIDE TO USE THIS STANDARD, Note 4 and example. Modified concrete classification. Modified "ALUMINUM COLUMN (POST) SELECTION TABLE".
5201	1 of 1	Texture Type "I" (Cut Coral Block) added.		2 of 8	Changed maximum limits of sign cluster area and width in NOTE.
202	1 of 4	Added precast post cap; Changed clearance tolerance on stepped panel and Neoprene Pad options.		3 of 8	Added Aluminum Soil Plate details and notes. Changed Post and Foundation Table depth values.
	3 of 4	Changed #4 Bar Mark to Bars P5 and P6 for Pile/Post Options A, B, & E; changed Texture Thickness to $1^1/4$ " Max.		4 of 8	Modified "ALUMINUM COLUMN (POST) SELECTION TABLE".  Deleted "Signs at 90°" note. Added "*For" note. Changed number of Z-brackets for STOP and RECTANGULAR sign. Changed '1" Min.' to '0" Min.' and sign paneledge distance in VIEW A-A. Modified U-bolt size. Changed paneloverhang length.
5203	1 of 5	Added precast post cap; Changed clearance tolerance on stepped panel and Neoprene Pad options.		5 of 8	Modified "DRIVEN POST DETAIL IN CONCRETE".
	3 of 5	Changed #4 Bar Mark to Bars P5 & P6 for Pile/Post Options A, B & E, and changed texture thickness dimension to $^{1}\!/_{\!4}$ " Max.	17302	1 of 1	CASE II, and CASE VIII dimensions and notes revised.
	4 of 5	New sheet added for 45 degree corner post.	17328	1 of 1	Weigh Station and combination Weigh Station and Inspection Station signing details separated.
	5 of 5	Renumbered from Sheet 4 of 4.			

Index Number	Sheet Number	Description	Index Number	Sheet Number	Description
17344	2, 3, 4 & 6 of 6	SCHDDL SIGNS AND MARKINGS, on each sheet, in the Distance table at the bottom of the sheet, deleted the "A" column. Also deleted the "A" dimension from the detail drawings.	17725	1 of 2	Round pole note revised; pole height dimensions added to Type P-III through P-VIII; Copper Ground note changed.
17345	2 of 4	NORMAL TAPERED ENTRANCE WITH ADDED LANE, note in lower left corner, arrow now points to the		2 of 2	Notes revised and rearranged, D(feet) changed to H(feet) in both tables.
	4 of 4	reflective markers on the LEFT side of the ramp.  Deleted note 2	17727	1-2 of 2	Schedule 40 aluminum pipe (T6061) added as an alternate to stainless steelpipe in assembly details and signal head notes. Added backplates to signal head details.
17346	1-14 of 14	Completely revised and renumbered.	17736	1 of 1	Added notes 5 & 6.
17347	1-4 of 4	New Index BICYCLE MARKINGS added.	17743	1 of 3	Updated assembly dimensions. Changed drilled shaft reinforcing.
17349	1 of 1	Case I and Case II revised; 18" x 18" marker detailrevised; notes at bottom right revised.		2 of 3	Updated assembly dimensions. Changed drilled shaft reinforcing. Changed T3-BF.
17355	1 of 11	Revised signs FTP-9A-06 & FTP-9B-06 and notes.		3 of 3	Updated assembly dimensions. Changed drilled shaft reinforcing.
	7 of 11	For all signs with 1-800 phone number, deleted "1-800-998-RIDE" and substituted "1-8XX-XXX-XXXX" and below each sign added note: "Design Project Manager or Transit Administrator will supply correct 1-8XX number".	17745	1 of 5 2 of 5	QPL requirements added in new note 17; added backplates to pole detail; Notes 6 & 14 revised, deleted note 19.  Revised foundation reinforcing details, Section AA, Section DD and Foundation Plan details.
	8 of 11	Revised sign FTP-68A-06, bolt holes located outside of sign message, notes revised. Sign FTP-69-06 and FTP-68B-06 message and spacing revised.	17748	1 of 1	Option 1 deleted and Options 2 and 3 renumbered; Note 1 revised. Added backplates to signal head displays.
	9 of 11	Revised sign FTP-82-08 and arrow detail. Added Sign FTP-83-08.	17784	1 of 2	Dimensions revised on Figures A & B. Note 5 and Note to Designers revised.
17356	1 of 1	Removed signal head from detail. Single point attachment details deleted from Index. (Deleted sheet 1.)		2 of 2	Revised details and spacing for signs FTP-68A-06 and FTP-68B-06, also located bolt holes outside of sign message.
17359	1 of 2	Changed delineators to object markers; revised reference notes; sign W13-1 made optional.	17890	2-3 of 3	Added backplates to signal head displays.
	2 of 2	RURAL NARROW BRIDGE TREATMENT, changed the OM3L on the right side of the roadways to an OM3R. Notes revised; inserts reorganized	17900	7 of 7	Changed pole type callouts, deleted "N-III" and substituted "P-III".
17500	1 of 3	Deleted concrete pole detail, added METAL POLE DETAIL AND WIRING DIAGRAM.	18111	1-2 of 2	Index totally revised.
	2 of 3	Note 7, deleted "class I Concrete (Miscellaneous)" replaced with "Concrete and reinforcing for slabs around poles and pull boxes shall be included in the price for pull box or pole."	18113	1-2 of 2	Index totally revised.
	3 of 3	Note 7, deleted "class I Concrete (Miscellaneous)" replaced with "Concrete and reinforcing for slabs around poles and pull boxes shall be included in the price for pull box or pole."	20110	1 of 1	Changed Insert Detail for Diaphragm Reinforcing.
17501	1 of 1	Deleted note 28.	20199	1 of 1	Changed BEAM CAMBER AND BUILD-UP NOTES.
17502	3 of 7	Changed Note 9. Added Notes 10 & 11. Changed Notes 11 & 12. Deleted grout pad notes (former	20210	2 of 2	Added "Type Q" Epoxy to Note 9.
	4 - 5 7	Notes 4 & 9). Added CSL tube note (Note 11).	20299	1 of 1	Changed BEAM CAMBER AND BUILD-UP NOTES.
	4 of 7 5 of 7	Added ID plate and changed base plate thickness. Deleted grout pad. Changed drilled shaft reinforcing.  Changed Weld symbol in SECTION A-A. Added padlock tab to HANDHOLE RING. Added Section E-E	20500	1 of 1	Added Type C Pads for larger skew ranges. Changed specification of elastomer from "durometer" to "shear modulus".
		detail and bottom baseplate washer to SECTION C-C. Deleted grout pad and added wire screen.  Added CSL tubes.	20501	1 of 1	Changed Note 4.
	6 of 7	Grout notes and details removed, new wire screen.	20502	1 - £ 1	Channel Nata 4
17503	7 of 7 1 of 1	Note 3, changed "Concrete class" to "concrete NS"  Index deleted.	20502	1 of 1	Changed Note 4.
17504		Dimensions 5'-6" added for height of meter base. Pole type changed from type "N" to type "P".	20602	1 of 1	Changed EDC location to 1D from tip of pile.
17504	1 of 1 1 of 2	Mercury Vapor Luminaires changed to Induction Luminaires. Luminaire chart deleted, dimensions revised	20900	2 of 2	Changed coping width and End Bent lug from 6" to $5\frac{1}{2}$ " thickness.
17515	1 of 8	on spacing detail note and added to structure detail.  Added median barrier mounted light poles. Moved notes to sheet 2.	20910	2 of 2	Changed coping width and End Bent lug from 6" to $5\frac{1}{2}$ " thickness.
1,010	2 of 8	New Sheet for Notes. Change Note 7 for QPL Criteria. Modified concrete classification. Added notes	21100	1 of 3	Deleted redundant notes from Specification Section 458.
		for median barrier mounted light pole and foundation.		3 of 3	Changed Sidewalk Cover Plate edge treatment.
	3 of 8 4 of 8	Sheet renumberd from 2 to 3. Added double arm configuration to ARM ELEVATION.  Allowed fusion weld reinforcing cage (*) and changed foundation concrete note. Added 1" dimension to Double Nuts in FOUNDATION. Modified concrete classification. Renumbered sheet from 3 of 3 to	21110	1 of 2	Deleted redundant notes from Specification Section 458. Changed last line of title of bottom left detail to "DECK WITH SLOPES 2% DR GREATER".
		4 of 8.		2 of 2	Changed Sidewalk Cover Plate edge treatment.
		New Sheets for median barrier mounted light pole.	21200	1 of 2	Added "Anchor Plate (dashed lines) (provide Design) to ELEVATION VIEW and TYPICAL SECTION.
17600	2 of 3	Added detail for pole foundation to be used only behind guardrail.		2 of 2	Added design of anchor bolts and accessories.  Added design of anchor bolts and accessories.
	3 of 3	GENERAL NOTES, note 2, changed "Class II Concrete" to "Class I Concrete"; changed note 4.			
17723	1 of 3	Changed Note 5i, 6 and 7. Added Note 8. Deleted grout pad and notes (former Notes 4d & 7). Added CSL tube note (Note 9).	21600	1 of 7 3 of 7	Clarified INSTRUCTIONS TO DESIGNER for variable end span lengths.  Added vertical dimensions between deck surface and underside of bearings, including depth of Truss
	2 of 3	Changed number of bolts in VIEW B-B, number and size of foundation reinforcing bars, and TABLE	21802	1 of 1	Panel.   Changed "Methyl Methacrylate" to "High Molecular Weight Methacrylate".
		OF STRAIN POLE VARIABLES. Added foundation standoff distance and washer for base plate. Deleted grout pad and added wire screen. Added CSL tubes. Changed drilled shaft reinforcing.	21803	1-2 of 3	Revised call—outs for Grout Dutlets; Changed "Methyl Methacrylate" to "High Molecular Weight Methacrylate".
	3 of 3	Changed note in VIEW E-E; Added $^{1}\!/_{4}$ " and $^{3}\!/_{8}$ " cable clamps and changed weld criteria. Changed clevis size.		3 of 3	Shrink wrap deleted from Duct Coupler Detail. Revised call—outs for Duct Couplers; Changed "Methyl Methacrylate" to "High Molecular Weight Methacrylate".

Br. D Degree Of Curvature, Depth, Density, Distance, Diameter Area or Amperes Bridge AAABrg. American Automobile Association or Directional Distribution Bearing AADT DA Annual Average Daily Traffic Brkwy. Breakaway Drainage Area or Deflection Angle AASH0 DBH Diameter At Breast Height American Association Of State Highway Officials ΒT Buried Telephone Cable or Duct **AASHTO** DBI Ditch Bottom Inlet American Association Of State Highway And Transportation Officials Btfly. Butterfly ABCAsphalt Base Course Dbl. Double BWBarbed Wire, Bottom Width or Both Ways Abd. DCS Degree Of Curvature (Spiral) Abandoned ABS DΩ Dry Density Acrylonitrite-Butadiene-Styrene Pipe Cantilever Length, Cut, Colorless, Coulomb or Cycle Length Directional Design Hour Traffic AC, Ac. ° C DDHVAcre Degree Celsius AC or Asph. Conc. Asphaltic Concrete Decel. Deceleration C & G Curb And Gutter Accel. Deg. Degree Acceleration CACoarse Aggregate Delineators ACIAmerican Concrete Institute Capacity Delin. Сар. Act. CAP Demobl. Demobilization Actuated Corrugated Aluminum Pipe ADADept. Department The Americans With Disabilities Act Caps. Capital Letters Adh. Detour, Detection, Detectable CASP Det. Adhesive Corrugated Aluminized Steel Pipe Adi. Adiust CATVDFE Design Flood Elevation Cable Television DGN or Dgn. ADTAverage Daily Traffic CBCatch Basin Design AFAD DHVDesign Hourly Volume Automatted Flagger Assistance Device CBC Concrete Box Culvert Agg. DHWDesign High Water CBS Aggregate Concrete Box Structure DΤ Ah. Ditch Ahead CC, C/C, C to C, or C.C. Center to Center, Crash Cushion **AISC** DIAmerican Institute Of Steel Construction CCEWCenter to Center Each Way Drop Inlet Alt. Alternate Dia. or D Diameter CCTVClosed-Circuit Television AI. Dim. Dimension Aluminum CDCross Drain, Cross Direction (Geotextiles) AM12:00 Midnight Until 11:59 Noon Disp. Disposal cd Candela **ANSI** Dist. Distance American National Standards Institute Cem. Cement or Cemetery ADS Apparent Opening Size DLS District Location Surveyor Cem'd. Cemented Appl.. Applied, Application Cubic Feet Per Second DMMDomestic Mail Manual CFS DOT Apprh. Department Of Transportation Approach Ch. Channel DPI or D.P.I. Ditch Point Intersection Approx. *Approximate* Chchq. Channel Change ARTBA American Road & Transportation Builders Association Chg. Changeable Dr. or DR. Drain, Drive or Design Review DR Design Review Artf. Artificial CICast Iron Asph. Asphalt Driv. Driven CIPCast Iron Pipe Assem. Assembly CIPL, C.I.P., C-I-P Drwy. Driveway Cast In Place DS Association Design Speed Assn. Circumference circ. DSL Assoc. Associate, Association Ckt. Circuit Design Service Life ASTM American Society For Testing And Materials Dwg. Drawing Cl. or Clear Clearance ATPB Asphalt Treated Permeable Base CL, C/L or C Center Line Ε East or External Distance Attn. Attention CMConcrete Monument Rate Of Superelevation Attnuatr. Attenuator **CMB** Concrete Median Barrier End to End E to E Aux. or Auxil. *Auxiliar v* CMP Corrugated Metal Pipe EA or Ea. Each **CMPA** Corrugated Metal Pipe Arch Ave. Avenue EΒ Eastbound AWGAmerican Wire Gauge Co. County or Company EIA Electronic Industries Alliance AWS American Welding Society Col. Column El. or Elev. Elevation AzAzimuth Com. Commercial or Common Elast. Elastomeric CDMMCommittee or By Committee Electric Elec. B to B Back to Back Comp. Composite Ellip. Elliptical Basc. *Bascule* Connect or Connection Con. Embk. Embankment Bd. or Bnd. Bond or Bonded Conc. Concrete Emulsified Emul. BCBottle Cap or Bolt Circle Const. Construct or Construction Encl. Enclosure Back Of Curb *B/C, B.C.* Contrl. Controller Engr. Engineer **BCCMP** Bituminous Coated Corrugated Metal Pipe Culvert Cont. Continuation EOS End Of Survey or Equivalent Opening Size *BCPA* Bituminous Coated Pipe Arch Culvert Contr. Contractor E.P. or EOP Edge Of Pavement **BCPCMP** Bituminous Coated And Paved Corrugated Metal Pipe Culvert Coordinate Coord. **EPDM** Ethylene Propylene Diene Monomer **BCPPA** Bituminous Coated And Paved Pipe Arch Culvert Cor. Corner Eq. Equation or Equal BCT Breakaway Cable Terminal Corr. Corrugated Equip. Equipment **BCWE** Base Clearance Water Elevation CP Concrete Pipe Esmt. Easement ΒE Buried Electric CPE Corrugated Polyethylene Pipe Est. or Estm. Estimate CPTCone Penetration Test Beg. Begin Establish or Established Est. CR Bit. Bituminous Control Radius or County Road Etc. or etc. Et Cetera (And So Forth) CRA Bk. Back Clear Recovery Area ETPElectronic Tough Pitch BL, BLC, or ₽ Base Line, Base Line Control Crs. or Cse. Course ΕW Endwall Buildina Curve To Spiral Bldg. CS Ex. Except, Example Bulkhead CSP Corrugated Steel Pipe Blkhd. Exc. or Excav Excavation BLON Begin Length Of Need CTClear Trunk Exist. Existing Boulevard CTPB Cement Treated Permeable Base Blvd. Ехр. Expansion ВМ Bench Mark Ctlvr. Cantilever Extension Ext. Ctr., Ctrs. Bndry. Boundary Center Exwy. Expressway Bdr. Border CU or Cu Copper Bot. Bottom Culv. Culvert *B0* Basin Outlet Cwt. Hundredweight The abbreviations listed are the standard for contract plans production. This list is not all BOS Beginning Of Survey CY,Cu. Yd., CY, or C.Y. Cubic Yard inclusive. Other Department accepted abbreviations may be used when deemed more appropriate. BP Borrow Pit Cylindrical Cyl. Where special abbreviations are used a descriptive tabulation may be necessary in the plans. Ва. Becquerel

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F	Fill, Farad	HW or H.W.	High Water or Hot Water	М	Mass, Middle Ordinate Length or Mega	N m	Newton Meter
F or Final	Final Quantity	Hwy.	Highway	$m_{_{2}}$	Meter or Milli	No.	Number
F & I	Furnish & Install	Hyd.	Hydraulic	$m^2$	Square Meter or Meter Square	Nom.	Nominal
F to F	Face to Face	Hz	Hertz	$m^{3}_{\underline{}}$	Cubic Meter or Meter Cubed	Norm.	Normal
FA	Federal Aid or Fine Aggregate			$m^3 / m$	Cubic Meter Per Meter	N.P.	Non Plastic
FAC	Florida Administrative Code	I	External Angle (Delta), Interstate	m/s	Meters Per Second	NPS	Nominal Pipe Size
FAP	Federal Aid Project	Intchg. or Ichg.	Interchange	Mach.	Machine	NPT	National Pipe Thread
FC	Friction Course	IES	Illuminating Engineering Society	Maint.	Maintenance	NRCP	Non-Reinforced Concrete Pipe
FD	French Drain	ID, I.D.	Inside Diameter or Identification	Matl.	Material	NS	Non Stress, Not Suitable or Near Side
Fdn.	Foundation	IMC	Intermediate Metal Conduit	Мах.	Maximum	NT, N&T	Non Traffic, Nail & Tin
FDOT	Florida Department Of Transportation	In.	Inch or Inches	MB	Median Barrier	NTS	Not To Scale
FE	Floor Elevation	Inc.	Incorporated or Including	MBM	Thousand (Feet) Board Measure	NW	Northwest
Fed.	Federal	Incl. or Inc.	Included	MD	Machine Direction (Geotextiles)		
Fert.	Fertilizer	Ind.	Industry or Industrial	Med.	Median	Opass	Overpass
FES	Flared End Section	INV. or Inv.	Invert	Mega	One Million	0 to 0, o to o or 0.0.	
FETS	Flared End Terminal Section	IP	Iron Pipe	Memb.	Member	OA .	Overall
FH	Fire Hydrant	Install.	Installed	MES	Mitered End Section	O.B.G.	Optional Base Group
FHWA	Federal Highway Administration	Isect.	Intersection	Mess.	Message	DC or D.C.	On Center
Fig.	Figure	Isl.	Island	Mfg.	Manufactured or Manufacturer	OD or O.D.	Outside Diameter
Fin.	Finish	IR	Iron Rod	МĞ	1000 Gallons	OE	Overhead Electric
F.L., FL or E	Flow Line	ITE	Institute Of Transportation Engineers	MH, M.H.	Manhole, Mounting Height	OH, OHD or Ohd.	Overhead
FL, Fl. or Fla.	Florida	ITS	Intelligent Transportation Systems	MHW	Mean High Water	Opt.	Option, Optional or Optically
Flex.	Flexible	113	Intelligent Transportation Systems	μ	Micro	OT	Overhead Telephone
FNQ	Fuse (Type Slow Burn)	J	Joule	Mi.	Mile	Oz.	Dunce
FOC	Fiber Optics Cable	JB	Junction Box	Micro	One-Millionth	Ω	0hm
FPM or fpm	Feet Per Minute	Jct.	Junction	Mid.	Middle	P	Passenger Car & Light Delivery Truck
FPS or fps	Feet Per Second	Jt.	Joint	Mil	One-Thousandth Of An Inch	P or Plan	Plan Quantity
FR or Fr.	Frame	- **		Mil.	Military	Pa	Pascal
Frang.	Frangible	K	Design Hour Factor or Kelvin	Milli	One – Thousandth	Par.	Parallel
Freq.	Frequency	k	Kilo (prefix)	Min.	Minimum or Minute	Pa•s	Parallel Pascal Second
F.S.	Florida Statutes	kg	Kilogram	Misc.	Miscellaneous	Part.	Participation or Partition
Ft.	Foot or Feet	kg/m	Kilogram Per Meter	mL	Milliliter	Part. Pavt.	Participation of Partition Pavement
FTB	Floating Turbidity Barrier	kg/m²	Kilogram Per Square Meter	ML W	Mean Low Water	PC	Point Of Curvature
FTBA	Florida Transportation Builder Association	kg/m³	Kilogram Per Cubic Meter	mm	Millimeter	PCBC	Precast Concrete Box Culvert
FTP	Florida Traffic Plans	Kilo	One Thousand	Mobl.	Mobilization	PCC	
Furn.	Furnish	Kip	1000 Pounds	Mod.	Modify or Modified	FCC	Point Of Compound Curvature or Plain Cement Concrete
T GITT.	i urriisti	km	Kilometer	Mod. Mol	Mole	PCE	Permanent Construction Easement
		km/h	Kilometer Per Hour	Mon.	Monument	PE PE	
G	Giga or Gauss	kn	Knot	MOT.	Maintenance Of Traffic	PE Ped	Professional Engineer
g	Gram or Gravity	kN	Kilonewton	MP	Mile Post		Pedestrian or Pedestal
Galv.	Galvanized	kPa	Kilopascal	MPa	Megapascal	Pen.	Penetration
Ga.	Gauge or Gage	ksi	Kips Per Square Inch		Miles Per Hour	PG PGL	Profile Grade
Ga. or Gal.	Gallon	kV	Kilovolt	MSL MSL	Mean Sea Level		Profile Grade Line
Gar.	Garage	kVA	Kilovolt Ampere	MSTCSD	Minimum Specifications For Traffic Control	Ph.	Phase
GD	Gutter Drain	k Wh	Kilowatthour	IVISTUSD	Signal Devices	pΗ	Measure Of Acidity or Alkalinity
GFI	Ground Fault Interrupter			Mtd.	Mounted	PI	Point Of Intersection
GIP	Galvanized Iron Pipe	L	Length, Length Of Curve, Liter, Left	MUTCD	Manual On Uniform Traffic Control Device	Pkg.	Parking
GM	Gas Main	2-L	Two-Lane	MUTS	Manual On Uniform Traffic Studies	Pkwy.	Parkway
GP	Grade Point	2L1W	Two-Lane One-Way	IVIUTS	Manual Uni Onitorni Traffic Studies	PL or P	Property Line or Plate
Gr.	Grade, Guardrail or Grate	2L2W	Two-Lane Two-Way	Ν	North or Newton	PM DBC	12:00 Noon Until 11:59 Midnight
Gr. or Gro.	Gross	LA or L/A	Limited Access		Newtons Per Meter	POC	Point On Curve
GRC	Galvanized Rigid Steel Conduit	Lat.	Lateral or Latitude	N/m	Newtons Fer Meter Newtons Per Square Meter	POST	Point On Semi-Tangent
Grd.	Ground	Lb.	Pound	N/m²		POT	Point On Tangent
GRI	Geosynthetic Research Institute	LBS.	Pounds	N/m³	Newtons Per Cubic Meter	PP	Power Pole
gross km	Gross Kilometer	lb/sy	Pounds Per Square Yard	N/mm²	Newtons Per Square Millimeter	PPB	Pier Protection Barrier
Gr. Wt. or gr. wt.		LBR	Limerock Bearing Ratio	NA or N/A	Not Available or Not Applicable	Pr.	Pair Disk 86 D
Gttr.	Gutter	LC	Long Chord	N & C N & D	Nail & Cap Nail & Disk	PRC	Point Of Reverse Curvature
		LEO	Law Enforcement With Flashing			Prost.	Precast
Н	Henry		Lights And Radar	NAVD	National American Vertical Datum	Prest.	Prestressed
h	Hour or Hecto	LFD	Load Factor Design	NB NC	Northbound	Prob.	Probability
ha	Hectare	Lgth.	Length		National Coarse or Normal Crown	Prod.	Product, Production, Producer or Produced
HAR	Highway Advisory Radio	Lin.	Linear	NCHRP	National Cooperative Research Program	Prog.	Program or Progression
HB	Hay Bales	lm	Lumen	NDCBU	Neighborhood Delivery And Collection Box Unit	Proj.	Project or Projection
HC	Horizontal Clearance	Lmrk.	Limerock	NE nation	Northeast Not Kilometer	PRM	Permanent Reference Monument
HD	High Density or Heavy Duty	LOS	Limit Of Clear Sight	net km	Net Kilometer	Prop.	Proposed
HD or Hd.	Head	Loc., LO	Location	NEMA	National Electrical Manufacturers Association	Prov.	Provisions
HDPE	High Density Polyethylene	Long.	Longitude	NGVD	National Geodetic Vertical Datum of 1929	PRS	Portable Regulatory Sign
Hdwl.	Headwall	LRFD	Load Resistance Factor Design	NGS	National Geodetic Survey	PS & E	Plans, Specifications And Estimates
HH	Heavy Hex	LS	Length Of Spiral	NHS	National Highway System	PSF or psf	Pounds Per Square Foot
Hndrl	Handrail	LT	Left Turn	NHW	Normal High Water	PSI or psi	Pounds Per Square Inch
HOA	Hand/Off/Automatic	Lt.	Left	NIC	Not In Contract	PT	Point Of Tangency or Pressure Treated
Horiz. or Hor.	Horizontal	Ltd.	Lighted or Limited	NJ	New Jersey	PVC	Polyvinyl Chloride
HP	High Pressure or Horsepower	Lum.	Luminaire			PW	Pressure Water
Hr.	Hour	L/W	Lightweight				
HS	High Strength	lx	Lux		0040 EDO:	F Design Standards	Last Oback No.
HSHV	High Strength Horizontal Vertical The abbi	***		tion SHIE OF	2010 FDO	Γ Design Standards	Revision Sheet No.
HSHV Hse.			re the standard for contract plans produc Other Department accepted abbreviations	tion.	<u> </u>		07/01/09 2 of 3
Hse. Ht.			d more appropriate. Where special abbrevi		■】 CTANDADD	<b>ABBREVIATIONS</b>	
116.			a more appropriate, where special abbrevi bulation may be necessary in the plans.		SIANDARD	ADDREVIATIONS	
	uie usec	, a acscriptive tu	outation may be necessary in the plans.	OF	RANK		001
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Q	Peak Discharge or Flow Volume	SRASP	Spiral Rib Aluminized Steel Pipe	V	Volt, Velocity, Volume or Hourly Volume	NITC C	DE MEACHDE
QPL	Qualified Products List	SRCP SRD	Steel Reinforced Concrete Pipe	Var.	varies, variable or variance		F MEASURE
R	Right	SRD SRSP	State Road Department SpiralRib SteelPipe	VC VCP	Vertical Curve Vitrified Clay Pipe	US MEASU	
R or Rad.	Radius	SS	Sanitary Sewer	VECP	Value Engineering Change Proposal	AC	Acre Assembly
R or Rng.	Range	SSMD	Solid State Modular Design	Veh.	Vehicle	AS BU	Bushel
rad	Radian	ST	Surface Treatment or Spiral To Tangent	Vert.	Vertical	CF	Cubic Foot
rad/s	Radian Per Second	St. or ST.	Street	VF	Vertical Foot	CD	Cleanout
RBAC RBST	Rock Base Asphaltic Concrete Rock Base Surface Treatment	Sta. Stab.	Station Stability or Stabilization	Vh VMS	Verified Horizontal Location	CY	Cubic Yard
RC	Reverse Crown	STB	Stability of Stabilization Staked Turbidity Barrier	VM3 Vol.	Variable Message Sign Volume	EA	Each
RCP	Reinforced Concrete Pipe	Std.	Standard	VP	Vertical Panel	ED	Each Day Gallon
RCPA	Reinforced Concrete Pipe Arch	Stg.	Strong	VPD or Vpd.	Vehicles Per Day	GA GM	Gross Mile
Rd.	Road or Round	Stge.	Storage	VPH or Vph.	Vehicles Per Hour	LB	Pound
Rdsd.	Roadside	Stl.	Steel		. Vehicles Per Hour Per Lane	LF	Linear Foot
Rdwy. Rec.	Roadway Recovery	Str. Sty.	Structure Story	VRMS V v	Volts Root Mean Square Verified Vertical Elevation	LM	Lane Mile
Rect.	Reticuline or Rectangular	SU.	Single Unit Trucks	Vvh	Verified Vertical Elevation And Horizontal Location	LO	Per Location
Ref.	Reference	Sub. or Subs.	Subsoil	VW	Variable Width	LS LU	Lump Sum Luminaire
Refl.	Reflective	Sub. or Subst.	Substitute			MB	Thousand Board Measure
Reg.	Region, Regular, Registered or Regulation	Subgr.	Subgrade	W	Width, Wide, West or Watt	MG	Thousand Gallons
Reinf.	Reinforced or Reinforcing	Suppts.	Supports	W/C	Water-Cement Ratio	MH	Man Hour
Rejuv. Reloc.	Rejuvenation Relocated	SUR or Sur. Surf.	Survey Surface	WB Wb.	Westbound Weber	NM	Net Mile
Rem.	Removal	SW	Southwest	WB40	Meber Intermediate Semi Trailer	PA	Per Analysis
Repl.	Replace	SW or Swk.	Sidewalk	WB50	Large Semi Trailer	PB PE	Per Building Pile
Req. or Reqd.	Required	Sys. or Syst.	System	WB62	Interstate Semi Trailer	PE PI	Per Intersection
Res.	Residence or Residential	Sv	Sievert	WB67D	Tandem Semi Trailer	PL	Plant
RGS	Rigid Galvanized Steel	Sym.	Symmetrical	WM W. D. T	Water Main	PM	Per Mile
RHW RM	Insulation (Moisture & Heat Resistant Rubber) Reference Monument	_		W.P.I. WT	Work Program Item Water Table Dr Weight	PS	Per Set
r/min	Revolution Per Minute	T TIMP Tour	Tangent, Length Of Curve, Percent Trucks, Tesla,	w / WWF	Water Table or Weight Welded Wire Fabric	PW	Per Well
RP	Reference Point	T, TWP or Twp.	Township Metric Ton	WWR	Welded Wire Reinforcing	SI SF	Square Inch Square Foot
rpm	Revolution Per Minute	tan.	Tangent		·	CV	Square Yard
RPM	Raised Reflective Pavement Markers	TBM	Temporary Bench Mark	X	Coordinate Value (East-West Direction) or Extra	TN	Ton
r/s	Revolution Per Second	TC	Tangent To Curve	X Rd.	Cross Road Crossing		EASUREMENT
RR RSDU	Railroad Radar Speed Display Unit	TCB	Temporary Concrete Barrier	Xing. Xsec.	Cross Section	AS	Assembly
Rsf.	Resurface	TCE TCP	Temporary Construction Easement	7,000.	0,000 00000	CO DA	Cleanout Day
Rt.	Right	TCZ	Terra Cotta Pipe Traffic Control Zone	Υ	Coordinate Value (North-South Direction)	EA	Each
RU	Rack Unit	TDLC	Transportation Design For Livable Communities	Yd.	Yard	ED	Each Day
R/W, ROW	Right Of Way	Tel.	Telephone	Yr.	Year	GK	Gross Kilometer
RX	Receive	Temp.	Temperature or Temporary			HA	Hectare
S or s	Speed, South, Siemens, Or Second	Theo.	Theoretical			HR KG	Hour Kilogram
SAHM	Sand-Asphalt Hot Mix	THRMPLSTC THW or THWN	Thermoplastic Insulation (Flame Retardant, Moisture And Heat Resistan	t Thermonlastic)		KL	Kiloliter
SAN or San. SB	Sanitary Southbound	Thick.	Thickness	( Thermopiastic)		KM	Kilometer
SBAC	Shell Base Asphaltic Concrete	Tk	Thick, Thickness or Truck			LI	Liter
SBRM	Sand Bituminous Road Mix	Tn.	Ton			LK	Lane Kilometer
SBST	Shell Base Surface Treatment	Traf.	Traffic			LO LS	Per Location Lump Sum
SC	Seal Coat or Spiral To Curve	Trans. Treat.	Transition, Transverse, Translate or Transportation Treatment				Lump Sum Per Assembly
Sch.	Schedule Sand-Clay Surface Treatment	TS	Treatment Tangent To Spiral			LS/DA	Lump Sum Per Day
SCST SD	Side Drain, Storm Drain	TSC	Length Of Tangent (Spiral Curve)				Lump Sum Per Each
SE	Southeast	TTC	Temporary Traffic Control				Lump Sum Per Hectare
Sec.	Second	TVSS	Transient Voltage Surge Suppression				Lump Sum Per Kilogram
Sect.	Section	TX	Transmit			LS/LS LS/MT	Lump Sum Per Lump Sum Lump Sum Per Metric Ton
Sed.	Sediment	Тур.	Typical			LS/MI	Lump Sum Per Linear Meter
Sep. Seg.	Separator Sequential	Upass.	Underpass				Lump Sum Per Square Meter
Seq. Serv.	Service	UG	Underground			LU	Luminaire
SF	Adjustment Factor In Percent, Silt Fence	UL	Underwriters Laboratories			MH	Man Hour
SG	Subgrade	Ult.	Ultimate			M□ MT	Month Metric Ton
SG	Specific Gravity	Ultd.	Unlimited			M1	Meter
Sh. or Sht.	Shoulder	Unddr. Undrdwy.	Underdrains Undergodway		The abbreviations listed are the standard	M2	Square Meter
Shldr. SHW	Shoulder Seasonal High Water	Unarawy. UNL or Undl.	Underroadway Unloaded		or contract plans production. This list is	M3	Cubic Meter
SIP	Stay In Place	Untr.	Untreated		not all inclusive. Other Department accepted	NK	Net Kilometer
SP	Superpave	UPS	Uninterruptible Power Supply	r	abbreviations may be used when deemed nore appropriate. Where special	PA BB	Per Analysis
Spa.	Space	USC & GS	US Coast and Geodetic Survey (now National Geodetic		ubbreviations are used a descriptive	PB PI	Per Building Per Intersection
Spcg. or Sp.	Spacing	USGS	US Geological Survey United States Postal Service		abulation may be necessary in the plans.	PL	Plant
Spec. SPT	Specification Standard Penetration Test	USPS Util.	United States Postal Service Utilities			PW	Per Well
Sq. Ft., SF, or S.F.		UV	Ultraviolet		2010 FDOT Design Standards		
Sq. In.	Square Inch				2010 1 DO 1 Design Granualus		Vealgion
Sq. Yd., SY or S.Y.					071115155 15555 15555 1555		07/01/07 3 of 3
SR or S.R. SRAP	State Road Spiral Rib Aluminum Pipe				STANDARD ABBREVIATIONS		Index No.
STAP	эрн анхи маннатн гіре		OF TRANS				001
						_	

# STANDARD SYMBOLS FOR KEY MAP

	Highway With Full Control of Access
	Highway With Frontage Roads
	Highway Interchange
'_	Proposed Controlled Access Highway
	Divided Highway
	Hard Surfaced Road
	Soil, Gravel Or Shell Surfaced Road
	Graded And Drained Road
	Unimproved Road
======	Primitive Road
	Private Road
	Streets In Inset Or Delimited Areas
	Extension Of LocalRoads Within Cities
FAI	Federal Aid Interstate Highway
<u>FAU</u>	Federal Aid Urban Highway
FAP	Federal Aid Primary Highway
FAS	Federal Aid Secondary Highway
NFR	National Forest Road
SFR	State Forest Road
SPR SPR	State Park Road
00	Interstate Highway
<u> </u>	US Numbered Highway
00	State Highway
(O)	County Road
<del></del>	Railroad
<del>-                  </del>	Double Track Railroad
+ + +	Abandoned Railroad
<del>-     <b>  -</b>     -  </del>	Railroad Station
	Grade Crossing
	Railroad Above
	Railroad Below
	Military Field
	Commercial Or Municipal Airport
$\sim$	Landing Area Or Strip

Runways

- FF	Free Ferry
TF-	Toll Ferry
	Canal Or Drainage Ditch
	Intracoastal Waterway
~~~	Narrow Stream
क्षाना सम्बन्धाः सम्बन्धाः सम्बन्धाः सम्बन्धाः । सम्बन्धाः सम्बन्धाः सम्बन्धाः सम्बन्धाः सम्बन्धाः ।	Wide Stream
	Dam
	Dam Or Spillway With Lock
	Dam With Road
	Flood Control Structure
	Lake, Reservoir Or Pond
	Intermittent Pond
M	Meandered Lake
	Marsh Or Swamp
14518 20 15 T	Mangroves
	Levee Or Dike
	Levee Or Dike With Road
<del></del>	Highway Bridge
<del></del>	Small Bridges Closely Spaced
<del></del>	Drawbridge
$\Longrightarrow \parallel \longleftarrow$	Highway Grade Separation
<u> </u>	Tunnel
	State Boundary Line
	County Boundary Line
	Civil Township Boundary
	Extended Township Line
	Land Grant Line
	Land Section Line
<del></del>	State Survey Section Line
<del></del>	Survey By Others
•••••	Location Of Inset Boundary Within Map
	Military Reservation Boundary
<u></u>	College Or University Boundary
777777777	Corporate Limits
	Delimited Area, Population Est.

Reservation, Forest Or Park Boundary Wildlife Refuge Boundary

	Residential Area Under Development		Agricultural Inspection Station
<b>\$</b>	Lighthouse	FM	Farmers Market
<b>★</b>	State Capital	$\underline{\mathbf{\Phi}}$	Game Preserve
	County Seat	<b>+</b>	Game Checking Station
0	Other City Or Village	<u> </u>	Bird Sanctuary
X	Seminole Indian Village		Fire Control Headquarters
$\stackrel{\wedge}{\sim}$	Welcome Station		Lookout Tower
WP	Wayside Park Or Small Park	FS	Fire Station
-WP	Park With Boat Ramp	*	Patrol Or Police Station
-B- -B-	Boat Ramp		Correctional Institution Or Road Camp
<b>=</b>	Museum	DOT	Department of Transportation Facility
<b>A</b> .	Recreational Area Or Historic Site		Coast Guard Station
П	Scenic Site	<b>W</b> A	Armory
	Post Office	J	Junkyard
	School	F	Sanitary Fill
<u>+</u>	Church	S	Sewage Disposal Plant
$\pm$	Cemetery	I	Incinerator
	Church And Cemetery	$\mathbf{z}$	Power Plant
4	Hospital, Health Center Or Rest Home	$\bigcap$	Power Substation
	Toll House, Port Of Entry Or Weight Station	<b>E</b>	Communications Facility
	Fair Grounds, Race Course Or Rodeo Arena	<del>-</del> XX-	Locked Gate Or Fence
	Mine Or Strip Mine	WOOD 📤	Triangulation Station
	Governmental Research Station		

#### GENERAL NOTE

1. Symbols on this Index are intended for use on all Roadway, Signing And Marking, Signalization, and Lighting projects. For work zone traffic control symbols refer to Index 600. When additional or similar symbols are used, legends or notations may be required for clarity.



2010 FDOT Design Standards

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Sheet No.

## STANDARD SYMBOLS FOR PLAN SHEETS

#### GENERAL SYMBOLS

#### = Curb — ---- County Line Curb And Gutter Water Well, Spring — Township Line Minimum Levee — — Section Line Railroad Mile Post City Line Railroad Signal With Gate — Base Or Survey Line --- Right-□f-Way Railroad Switch ———— Easement Line —<mark>≻•12° →</mark>— Gate —/-/-/ Limited Access Line 0 0 Pump Island Storage Tank (Surface) \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* National Or State Park Or Forest $(\Box$ Storage Tank (Underground) Grant Line × Mine Or Quarry ВР Borrow Pit ── Railroad (Detail Plans) † Church •••• Fence (Limited Access) Store Box Culvert Residence Bridge -─ Pipe Culvert-Mitered End Section School → Pipe Culvert-Straight Endwall Synthetic Bales —E Pipe Culvert-U-Type Endwall \_\_\_\_ Silt Fence — Pipe Culvert-Median Drain ✓ Pipe Culvert-Other End Treatments —18" SD—— Storm Drain (Proposed) Stream --- 18" SD----- Storm Drain (Existing) Shore Line علد علد علد Marsh ——◎— Manhole الم علم علم الله Wetland Boundary (Proposed) Tied Longitudinal Joint | Wetland Boundary (Existing عاد \_ عاد Keyed Longitudinal Joint — — — Hedge 습유 습유 Trees Doweled Transverse Expansion Joint HHHHHHHHHH Doweled Transverse Contraction Joint Community Edge Of Wooded Area — — — Transverse Contraction Joint Without Dowels <sup>ద</sup>ం<sup>దినిద</sup>ు Shrubbery $\oplus$ Survey Reference Point ALACHUA Triangulation Station Definition Of Skew For Cross Drains B.M. NO. 112 Bench Mark And Barrels Of Conrete Box Culverts Point Of Intersection Skew Lt. North Arrow TYP. Edges Of Existing Pavement And Sidewalk Concrete Crash Cushion (Attenuator) Rate Of Superelevation Piling Pier Column 0 Concrete Monument ₽ Base Line Centerline Flow Line Property Line

 $\triangle$ 

 $\pm$ 

Delta Angle

Approximate

Round Or Diameter

#### UTILITY ADJUSTMENT SYMBOLS

EXISTING	PROPOSED		EXISTING	PROPOSED	
6	0	Manhole	w 6'' m	w w w w w w 6 w w w w w w	Water Main
() EE3	₫ □	Fire Hydrant Meter (Type)	NPW 6" Man	ири ири 6'' маи маи	Non Potable Water
— t~~	— <del>—</del>	Valve (Type)	s 8" s	ssssss8" sssss	Sanitary Sewer
-67- -[4-	- <del> </del> ≥- 	Valve Box (Type) Valve Cover (Type)	G 6" 9	5 5 5 5 5 5 6 " 5 5 5 5 5 5	Gas
<b>○</b>	<b>○</b>	Vent (Type)	RD 4" da	ко ко "4 оя оя	Roof Drain
- <b>-</b>	<u></u>	Pump Station Sewage Pump Station	PET 8" 13d	PET PET 8" PET PET	Petroleum
		Cleanout	sтм 12" міs	sтм sтм 12" міs міs	Steam
<u>□</u>	—	Cable TV Service Box Power Pole	cas 12"svo	cas cas 12" svo svo	Casing
- ()— ·	<u> </u>	Telephone Pole	рт 4"х4" то	рт рт 4"х4" да да	Duct
- <b>&gt;</b> -		Combination Pole Guy Wire And Anchor Pin	ве (7.5 kV) эө	BE BE (7.5 kV) BE BE	Buried Electric
下. 下.文	$\stackrel{ o}{ \boxtimes}$	Guy Pole Deadman Tower	oe (7.5 kV) 30	30 30 (7.5 kV) DE DE	Overhead Electric
	$\circ$	Light Pole	вту 3"лів	вту вту3" вту вту	Buried Cable Television
·	W	Transformer	otv2" ^10	vio vio 5" vio vio	Overhead Cable Television
			вт 2'' 18	вт вт вт 2" тв тв тв	Buried Telephone
			от 2'' 10	от от от 2'' 10 10 10	Overhead Telephone
			вго 2"озв	BFO BFO 2" 038 038	Buried Fiber Optic
			ofo1" 030	OFO OFO 1" OFO OFO	Overhead Fiber Optic

See General Note, Sheet 1 of 3



2010 FDOT Design Standards

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#### STANDARD SYMBOLS FOR PLAN SHEETS

#### SIGNING AND PAVEMENT MARKING SYMBOLS TRAFFIC SIGNALS SYMBOLS LIGHTING SYMBOLS EXISTING PROPOSED EXISTING PROPOSED ()--() $\bigcirc$ Pole & Luminaire <del><</del> −<u></u>\_| Traffic Signal Head (Span Wire Mounted) Pavement Arrow Existing Pole & Luminaire To Be Removed $\bigcirc \times \bigcirc$ Traffic Signal Head (Pedestal Mounted) Single Solid Line ()----<del>|</del> Final Position Of Relocated Or Adjusted Pole & Luminaire Traffic Signal Head (Mast Arm Mounted) Double Solid Line $\bigcirc$ High Mast Lighting Tower Traffic Signal Pole (Concrete, Wood, Metal) Skip Line Vehicle Detector (Loop) X City Or Utility Owned Luminaire & Pole Stop Bar Signal Cable (On Messenger Wire) PVC (Polyvinyl Chloride) Lighting Conduit And Conductors Traffic Sign (Post Mounted) Conduit Rigid Galvanized Lighting Conduit And Conductors Traffic Sign (Overhead) (X)Vehicle Detector (Points) Lighting Pull-Box Sign Number Pedestrian Detector Light Distribution Point Sign Item Number Pedestrian Signal Head (Pole Or Pedestal Mounted) $\bigcirc$ Joint Use Pole Traffic Flow Arrow Controller Cabinet (Base Mounted) Pier Cap Underdeck Luminaire Controller Cabinet (Pole Mounted) Pendant Hung Underdeck Luminaire W - D WWalk - Dont Walk FDW Flashing Dont Walk 5 Signal Face Number Signal Lens P> Programmed Signal Head Messenger Wire **3** Pole Tabulation Cross Reference \*(3) Pole Tabulation Cross Reference (Joint Use Pole) $\varnothing$ Signal Phase

See General Note, Sheet 1 of 3



#### GENERAL NOTES

- 1. The illustrations for guardrail applications are standard configurations; adjustments are to be made as required by site specific conditions to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figures 1 and 2, and other application details of this Index.
- 3. One Panel (i.e., panel length) equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standards (indexes) or specifically called for in the plans.

Post spacing shall be 6'-3'' except that reduced spacing shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Detail J and Index No. 402) and transitions to redirective crash cushions, (b) the conditions in Note No. 7 below, (c) special post applications, (d) reduced post spacing required for specific end anchorage assemblies, and, (e) specific spacing called for in the plans.

- 4. Guardrail mounting height for the W-beam without rubrail and for thrie-beam is 1'-9" to the center of beam, and for W-beam with rubrail 2'-0" to center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a tolerance of 3" above and 1" below the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines).
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end treatments for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end treatments will be constructed only when restraints prevent construction of flared end anchorages.

Guardrail end anchorage assemblies shall be of the type called for in the plans. If the plans call for end anchorage assembly "flared" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDDT approved flared assembly provided in this Index or identified on the Qualified Products List (QPL), subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for end anchorage assembly "parallel" and does not identify the specific system(s) to be used, the contractor has the option to construct any FDOT approved parallel assembly provided in this Index or identified on the QPL, subject to the conditions identified in the approved Index drawings, or QPL drawings if applicable.

If the plans call for a specific end anchorage assembly, substitutions with other end anchorage assemblies will not be permitted unless approved by the Engineer. Approved substitutions will not be eligible for VECP consideration.

When an end treatment is attached to guardrail with Pedestrian Safety Treatment, only end treatment systems with timber posts are to be used.

Proprietary end anchorage systems must be identified on the QPL. Manufacturers seeking approval of proprietary end anchorage systems for inclusion on the QPL must submit application along with design documentation showing the end anchorage system is crash tested to NCHRP Report 350 Test Level 3 criteria, is accepted by FHWA for use as a guardrail end anchorage system, and is compatible with FDOT guardrail systems. System approvals will be contingent on FDOT's evaluation of crash test performance results for consistency with FDOT guardrail application and use. If approved, installation drawings signed and sealed by a professional engineer licensed in the State of Florida will be required.

- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 4'minimum for standard W-beam, other guardrail configurations may be applicable; see General Note No. 11 and the minimum offset table on Sheet 19. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier shall be used unless other shielding is approved by the Engineer of Record. See Index No. 410 for safety shape concrete barriers and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at roadside hazards or other areas where the Engineer has deemed guardrail necessary, guardrail should be considered on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone and fill heights are 6' or greater. Curbed sections where fill slopes are steeper than 1:3 and fill heights are 6' or greater within 22' of the traveled way should be evaluated for installation of guardrail. Additional guidance for evaluating the need for guardrail can be found in the Plans Preparation Manual.
- 9. The guardrail to bridge connections contained in this Index are for bridges with Test Level 4 traffic railing barriers. For guardrail to concrete barrier wall connections see Index No. 410. For existing bridges receiving retrofit traffic railing barriers see Index No. 402.
- 10. The W-beam guardrail system in this index is the standard system to be used on the State Highway System where a Test Level 3 semi-rigid barrier is required.
- 11. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic railing barriers, to concrete and certain water filled safety shaped barriers, certain crash cushions and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 402, 410 and 414. The use of thrie-beam guardrail with standard offset blocks (Test Level 3 semi-rigid system) may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
  - a. W-beam deflection is marginal,
  - b. W-beam with rubrail considered functionally deficient, continued ...

continued ...

- (c) Vehicle overriding W-beam is probable,
- (d) Drainage will be impeded or blocked by the use of concrete barrier wall (subject to deflection space requirements),
- (e) High frequency of repairs to W-beam,
- (f) Spandrelbeam with low deflection needed around unrelocatable structure,
- (g) Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g., passenger vans and small buses).

The modified thrie-beam guardrailis a Test Level 4 semi-rigid system and may be used where a Test Level 4 guardrail is required.

- 12. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
  - (a) Wide medians where approach end anchor is located outside of opposing roadway clear zone,
  - (b) Medians of uniform width that are occupied by other transportation and joint use facilities,
  - (c) Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail installations.
  - (d) Medians of bifurcated roadways.
- 13. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 14. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crashworthiness at terminals. Crash cushions shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 15. Corrugated sheet steel beams, end shoes, end sections and back-up plates shall conform to the current requirements of AASHTD M180, Class A, Type II (zinc) coating. All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTD requirements.

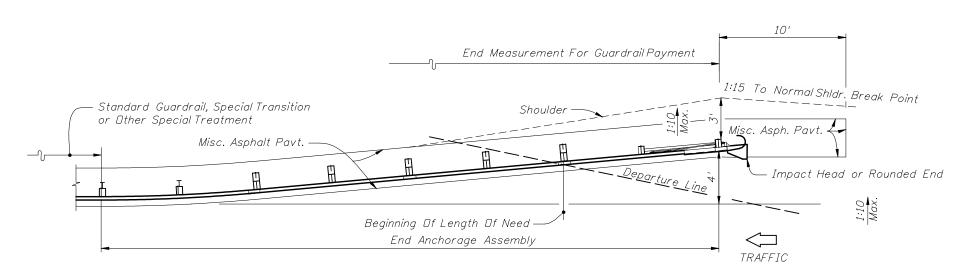
Recycled beams: Used Class A guardrail beams that have been refurbished to condition new (AASHTO M180) may be used for both construction of new guardrail and maintenance of existing guardrail. Refurbishing shall include stripping of the existing galvanizing, restoration of the base metal in section and straightness free of warp and deformation, and, regalvanizing to AASHTO Type II specifications. Refurbished beams that retain ruptured holes, gashes or tears will not be accepted.

- 16. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing steel offset blocks may remain throughout the service life of the existing guardrail. Permissible post and offset block combinations are tabulated on Sheet 16.
- 17. Where necessary to enlarge or add holes to galvanized guardrail, the work will be done by drilling or reaming.

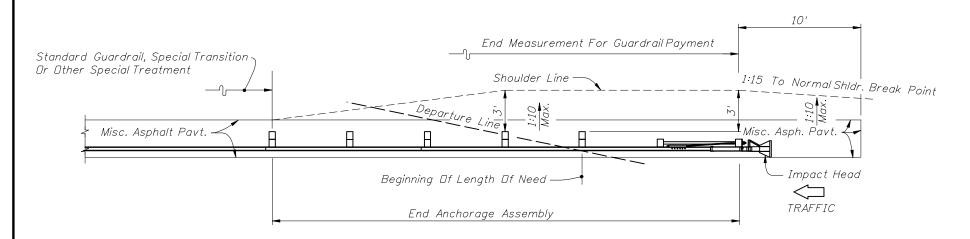
  Damaged galvanized guardrail will be metalized in accordance with Sections 562 and 975 of the Standard Specifications.

  No burning of holes will be permitted.
- 18. For guardrail reflector details see Sheet 17.
- 19. Any run of guardrail with existing concrete posts that is being reset under a construction or maintenance contract shall be reset using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 20. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for VECP consideration.
- 21. On roadways designated for reverse laning, all downstream ends of guardrail that are not shielded or that are not designed as approach end terminals shall be marked with post-mounted Type 3 Object Markers. Trailing bridge ends and trailing shoulder concrete barrier wall ends shall be marked with Type 3 Object Markers except where there is trailing end guardrail. Object markers to be installed facing reverse laning traffic. The cost of the object marker shall be included in the cost of the guardrail.

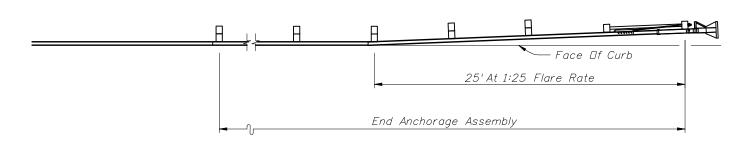




#### FLARED OPTION



PARALLEL OPTION



PLACEMENT OF PARALLEL OPTION AT CURBED LOCATIONS

#### GENERAL NOTES

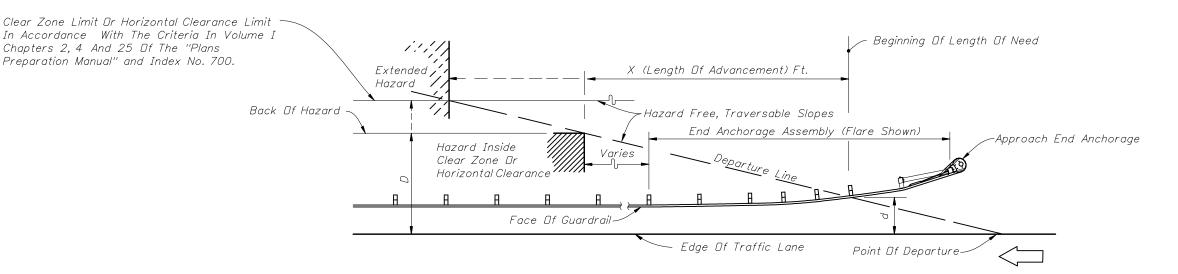
- 1. These drawings are representative of the various proprietary guardrailend anchorage assemblies listed on the Department's Qualified Products List (QPL). For specific details and requirements see the vendor drawings on the QPL at www.dot.state.fl.us/specificationsoffice/
- 2. These drawings present the general graphics to show the limits of payment for guardrail and end anchorage assemblies, modifications to the shoulder and placement of the miscellaneous asphalt mow strips.
- 3. These drawings, along with the various vendor drawings on the QPL, are intended to show sufficient details for installation of the end anchorage assemblies and their connection to shoulder guardrail. This precludes the requirement for shop drawing submittals unless otherwise called for in the plans. The various end anchorage assemblies shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.
- 4. The various proprietary end treatments listed on the QPL are intended for use as approach end guardrail anchorages for shoulder guardrail. The effective length of the end treatments vary-refer to the vendor drawings on the QPL for the length and the use of special panels and details. Standard guardrail, guardrail transitions or other special treatments shall not be included within the limits of the end anchorage assembly. See the vendor drawings for the alignment of the end treatment with respect to the normal guardrail alignment.
- 5. Flared or parallel end anchorage assemblies shall not be used in medians where horizontal clearance requires use of a back rail.
- 6. Each of the various end anchorage assemblies have unique features. Careful attention should be given to the types and orientation of the posts and other components. Refer to the vendor drawings on the QPL for the specific requirements of each system.
- 7. For galvanizing requirements of the metallic components see Standard Specifications Section 967.
- 8. The end anchorage assemblies shown on the QPL are suitable for all design speeds.
- 9. Flared end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Flared), EA.

Parallel end anchorage assemblies shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Parallel), EA.

The unit price for end anchorage assemblies shall be full compensation for furnishing and installing all components in accordance with the plans, the manufacturer's detail drawings, procedures and specifications and these drawings.

APPROACH END ANCHORAGE DETAILS





Design Speed mph	X (Length Of Advancement) Ft.
≤45	= 16 (D-d)
≥50	= 13 (D-d)

✓ Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.

The flared end anchorage with 4'nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

#### Equation Variables:

D=Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

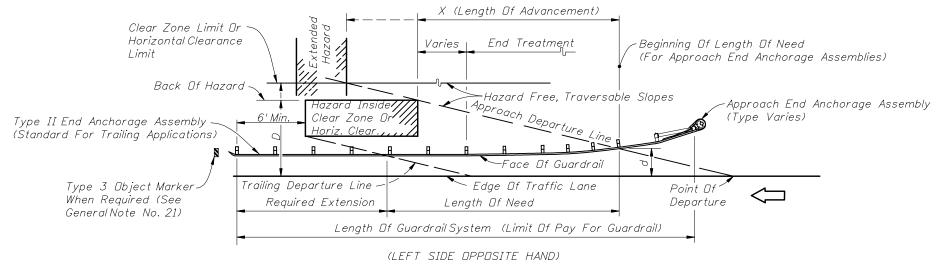
d=Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

For flared and parallel end anchorage assemblies the beginning length of need is to be set at the center of post #3. That is, the departure line must intersect the face of the rail at post #3.

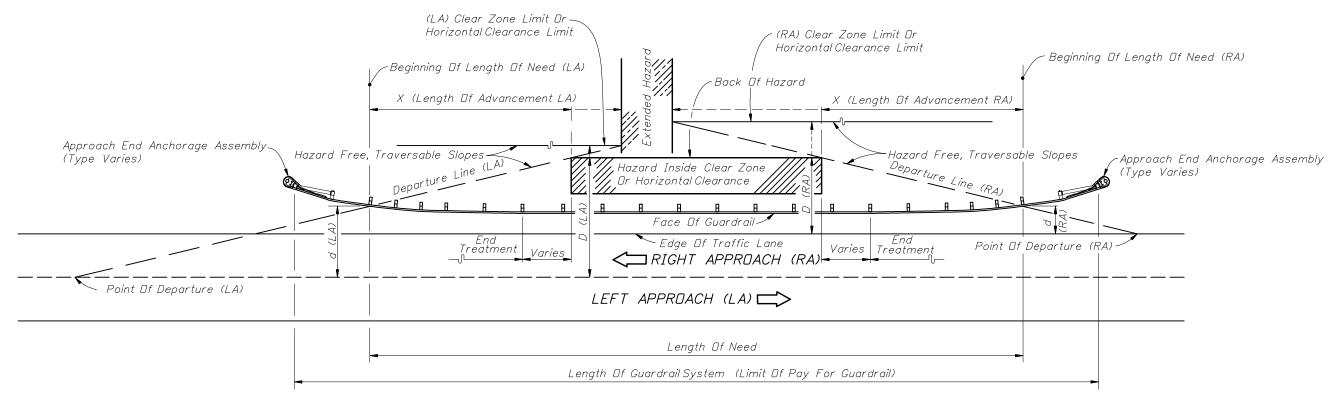
For flared end anchorage assemblies the offset distance "d" will equal the normal guardrail offset measured from the face of the guardrail to the edge of the near approach travellane plus 1'-2'' for 45 mph or less and  $1'-9\frac{1}{4}$ " for greater than 45 mph.

#### LENGTH OF ADVANCEMENT - FIGURE 1





## ONE-WAY TRAFFIC



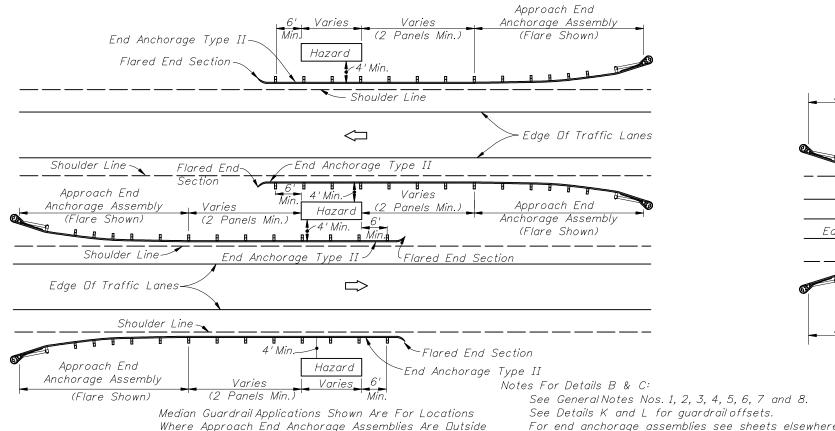
TWO-LANE TWO-WAY TRAFFIC

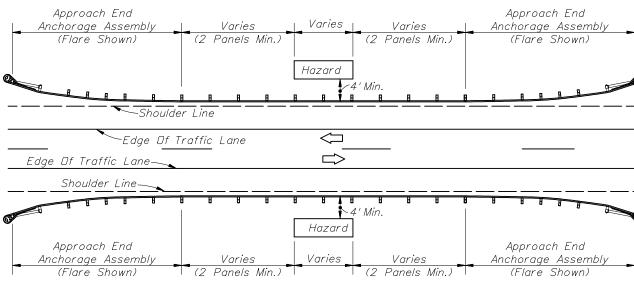
For description of the dimensions D, d and X, see Length of Advancement – Figure 1.

For additional shoulder quardrail information, see Details B and C.

LOCATING TERMINALS ON SHOULDER GUARDRAILS - FIGURE 2







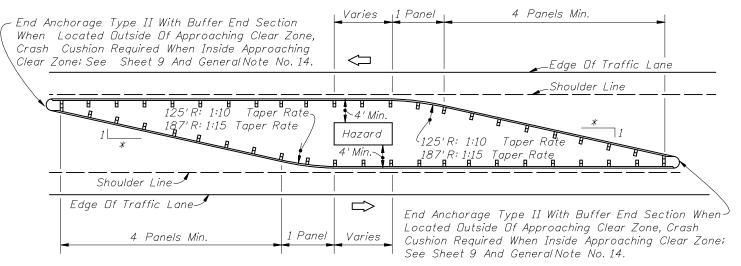
## UNDIVIDED ROADWAY- DETAIL C

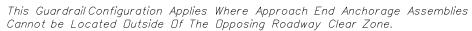
Of The Opposing Roadway Clear Zone. DIVIDED ROADWAY- DETAIL B For end anchorage assemblies see sheets elsewhere in this Index and the plans.

For hazards that require shielding and are located back of curb see other sheets of this index, and where

rigid barrier is required see Index No. 410.

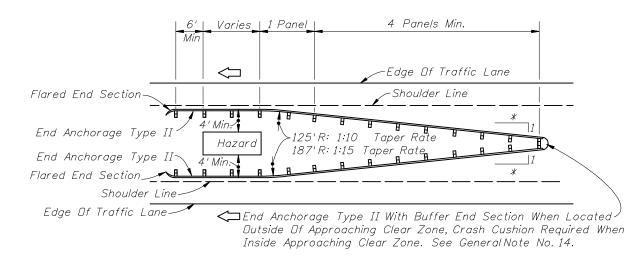
# GUARDRAIL APPLICATION FOR ROADSIDE HAZARDS





OPPOSING TRAFFIC- DETAIL D

Notes For Details D & G: See General Notes Nos. 1, 2, 3, 4, 5, 7, and 14. See Details K and L for guardrail offsets. For hazards that require shielding and are located back of curb see other sheets of this index, and where rigid barrier is required see Index No. 410.



#### ONE-WAY TRAFFIC- DETAIL G

\*1:10 Taper Rate For Design Speeds ≤45 mph 1:15 Taper Rate For Design Speeds ≥50 mph

GUARDRAIL APPLICATION FOR NARROW MEDIAN AND GORE HAZARDS

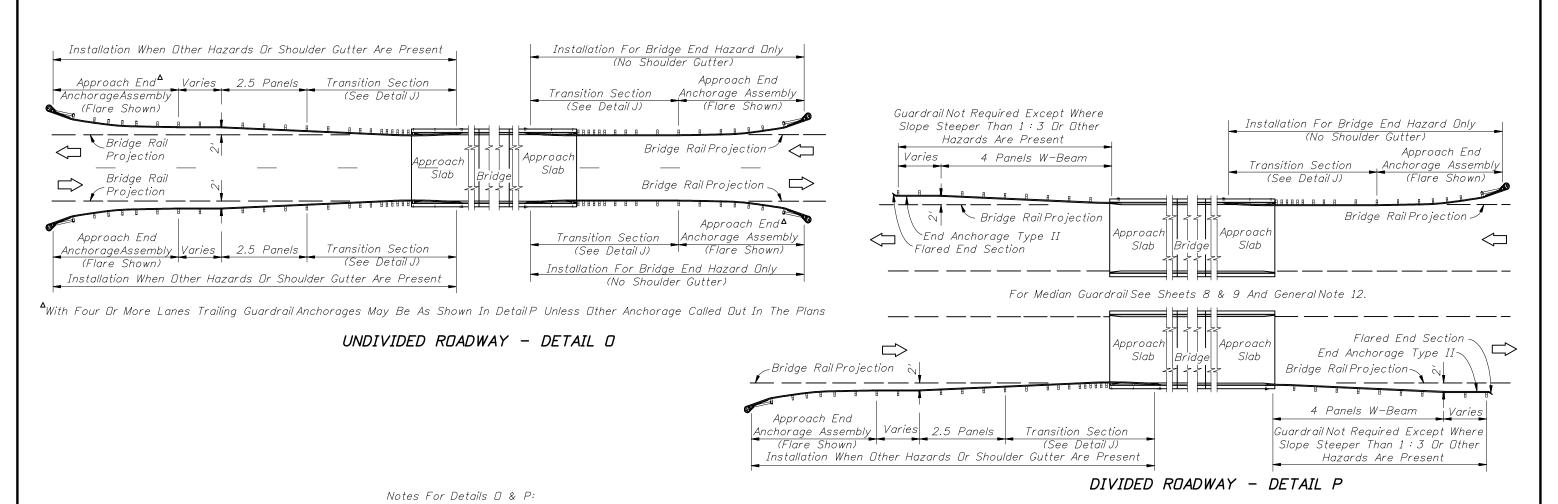


2010 FDOT Design Standards

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**GUARDRAIL** 

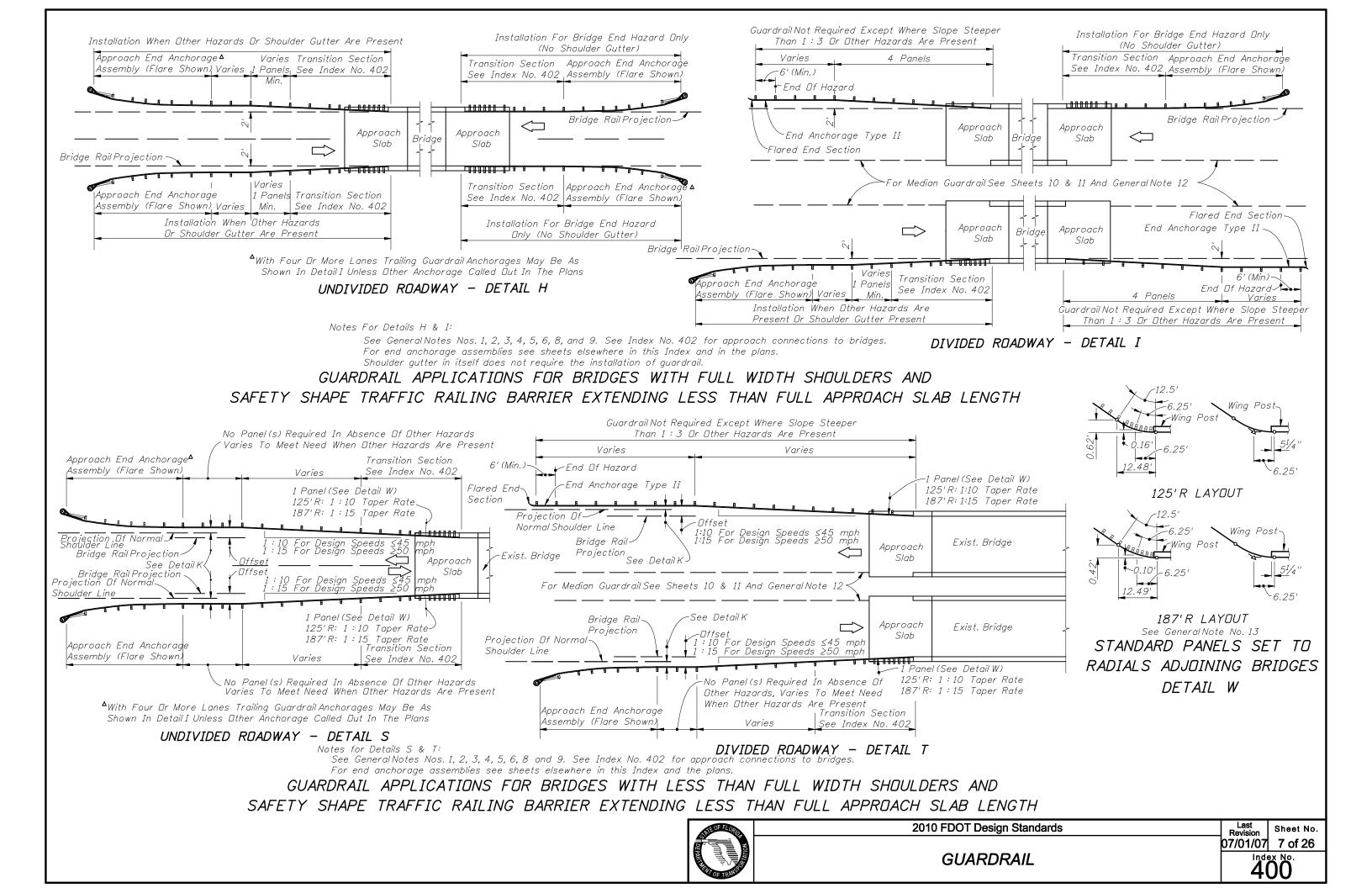
400°

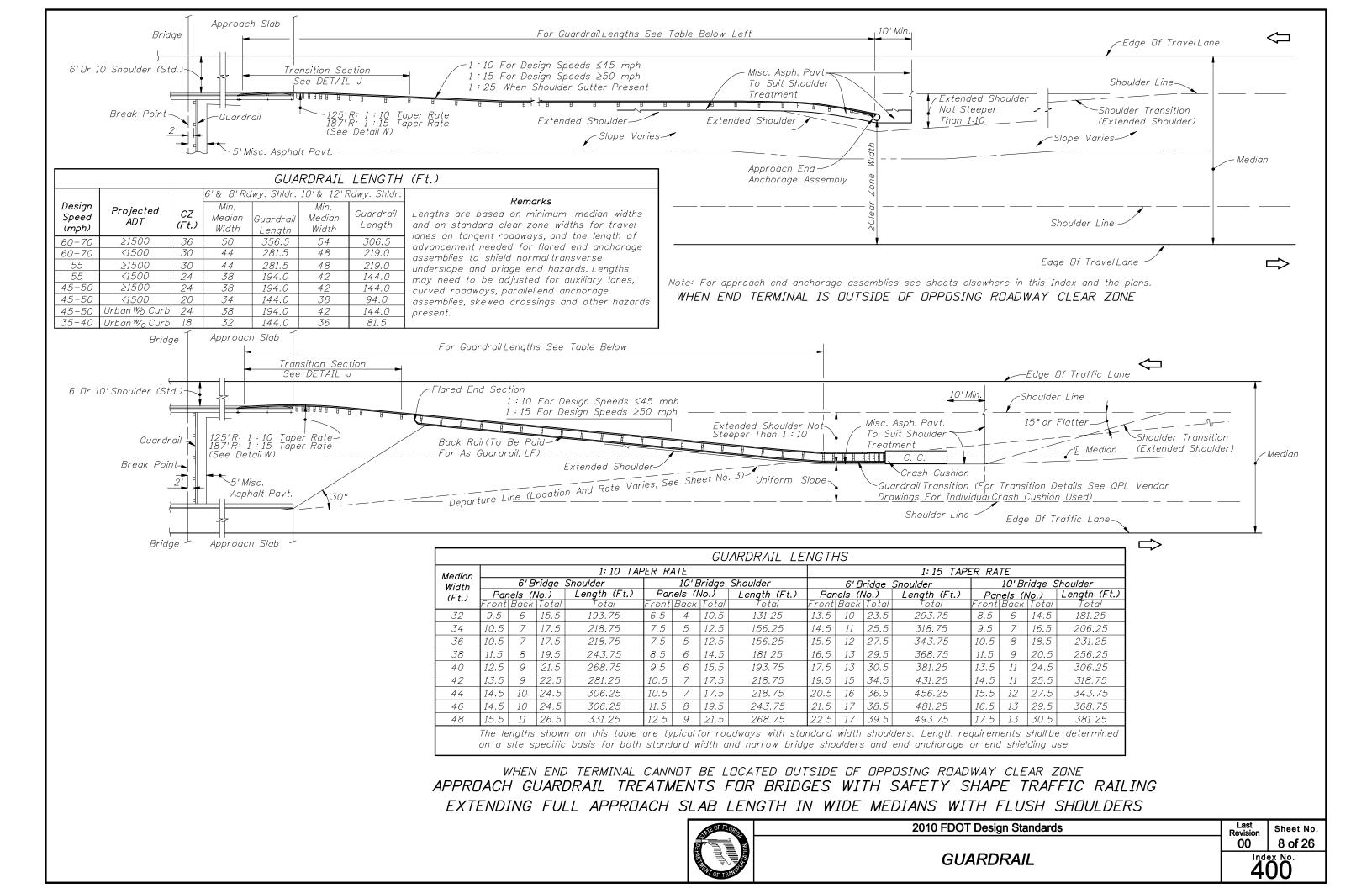


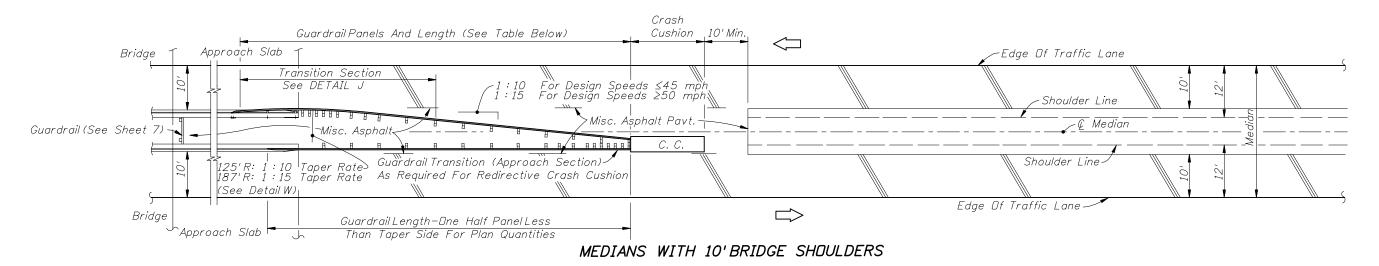
See General Notes Nos. 1, 2, 3, 4, 5, 6, 8 and 9. See Detail J for approach connections to bridges. For end anchorage assemblies see sheets elsewhere in this Index and the plans. Shoulder gutter in itself does not require the installation of guardrail.

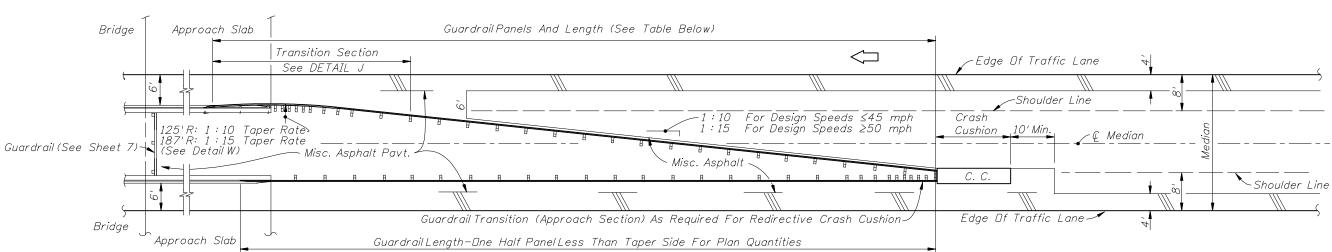
GUARDRAIL APPLICATIONS FOR BRIDGES WITH FULL WIDTH SHOULDERS AND SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING FULL LENGTH OF APPROACH SLAB





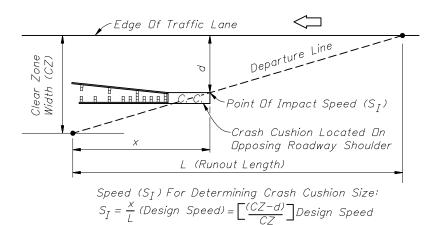






Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

MEDIANS WITH 6'BRIDGE SHOULDERS □>



SIZING CRASH CUSHIONS LOCATED
ON OPPOSING ROADWAY SHOULDERS

GUARDRAIL LENGTHS								
MEDIAN		6' BRIDGE	SHOULDERS	S	10' BRIDGE SHOULDERS			
WIDTH	1: 10 TAP	PER RATE	1: 15 TAP	PER RATE	1: 10 TAP	PER RATE	1: 15 TAF	PER RATE
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	14.5	181.25	20.5	256.25	7.5	93.75	10.5	131.25
28	12.5	156.25	18.5	231.25	6.5	81.25	8.5	106.25
26	11.5	143.75	15.5	193.75	5.5*	68.75	6.5	81.25
24	9.5	118.75	13.5	168.75	5.5 <i>*</i>	68.75	5.5 <i>*</i>	68.75

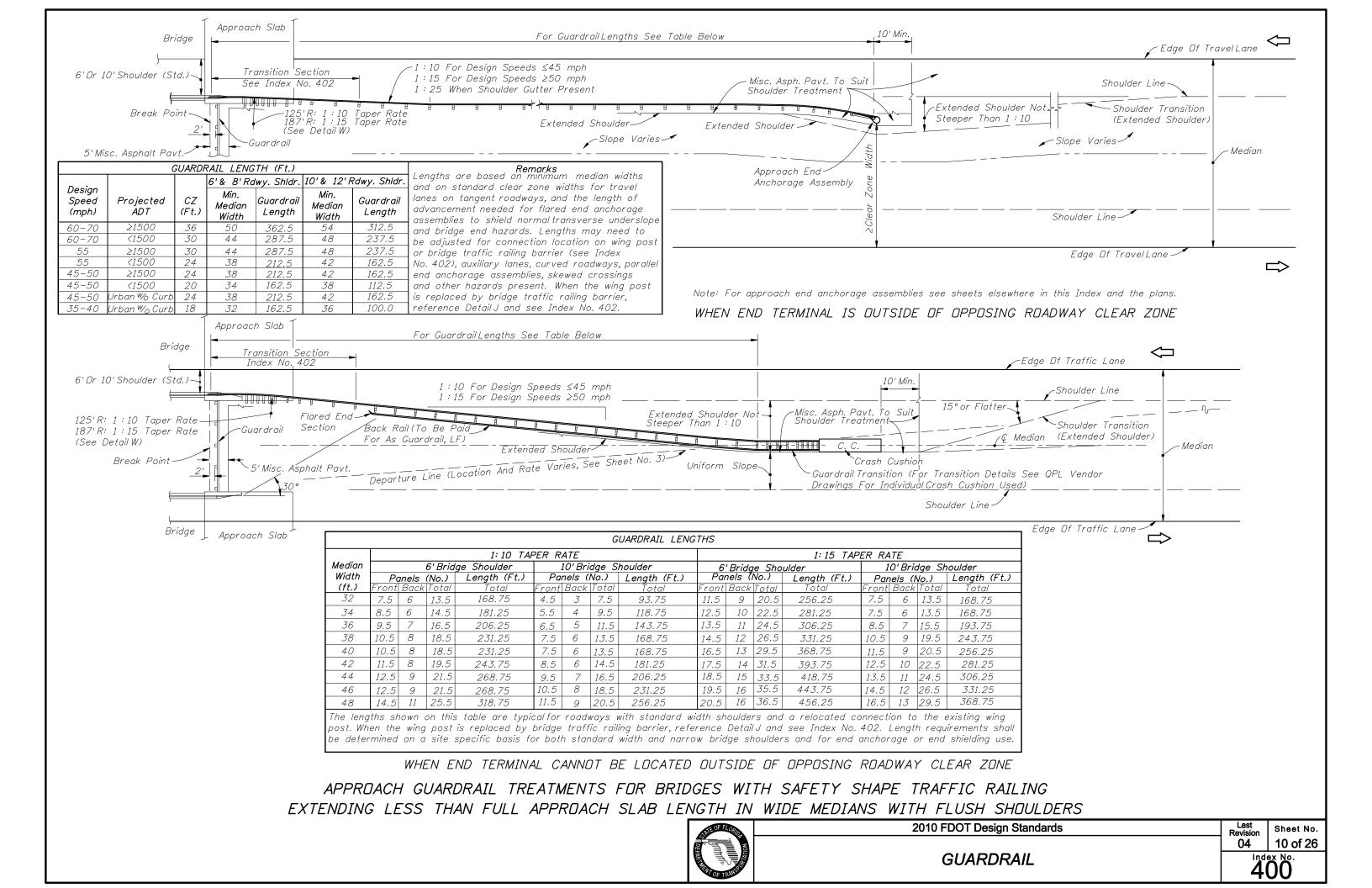
The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds  $(S_I)$  along the runouts from the approach roadways; however, when calculated speeds  $(S_I)$  are less than 30 mph; crash cushions shall be no less in size than for 30 mph, see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width, see \* below.

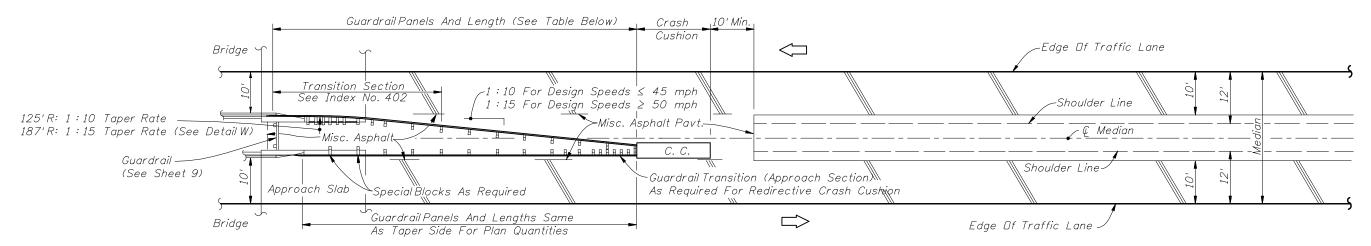
\* Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

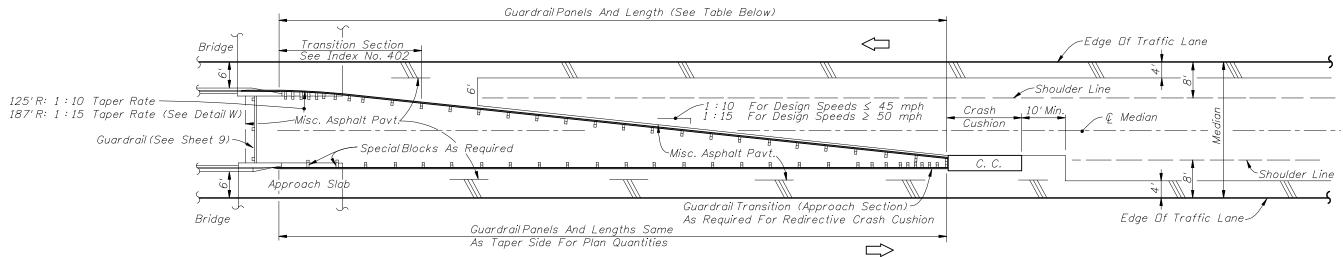


2010 FDOT Design Standards	Last Revision	Last Sheet No	
		9 of 26	
GUARDRAIL	Ind	No.	
		. / . /	





#### MEDIANS WITH 10' BRIDGE SHOULDERS



# MEDIANS WITH 6'BRIDGE SHOULDERS

Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

Edge Of Traffic Lane
Departure Line  Departure Line  Point Of Impact Speed (S <sup>I</sup> )  Crash Cushion Located On
Dpposing Roadway Shoulder
L (Runout Length)
Speed (S <sub>1</sub> ) For Determining Crash Cushion Size:

Speed  $(S_I)$  For Determining Crash Cushion Size:  $S_I = \frac{x}{L}$  (Design Speed) =  $\left[\frac{(CZ-d)}{CZ}\right]$  Design Speed

SIZING CRASH CUSHIONS LOCATED
ON OPPOSING ROADWAY SHOULDERS

	GUARDRAIL LENGTHS							
		6' BRIDGE	SHOULDERS	10' BRIDGE SHOULDERS				
MEDIAN WIDTH	1 1 1 1 APER RATE		1:15 TAPER RATE		1:10 TAPER RATE		1:15 TAPER RATE	
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)
30	12.5	156.25	18.5	231.25	6.5	81.25	9.5	118.75
28	11.5	143.75	16.5	206.25	5.5	68.75	7.5	93.75
26	9.5	<i>118.75</i>	14.5	181.25	5.5∦	<i>68.75</i>	5.5∦	68.75
24	8.5	106.25	11.5	<i>143.75</i>	5.5∦	68.75	5.5*	68.75

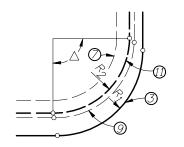
The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds  $(S_{\underline{I}})$  along the runouts from the approach roadways; however, when calculated speeds  $(S_{\underline{I}})$  are less than 30 mph crash cushions shall be no less in size than for 30 mph; see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see \* below.

\*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS



Sheet No. 11 of 26



GUARDRAIL

RADIAL

#### LEGEND

- ① Edge of traffic lane for simple curve turnouts.

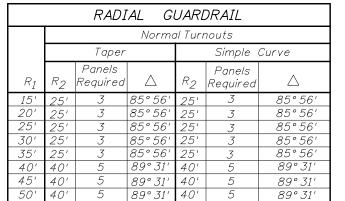
  Edge of travellane for taper turnouts.
- ② Taper.

Intersecting Drive

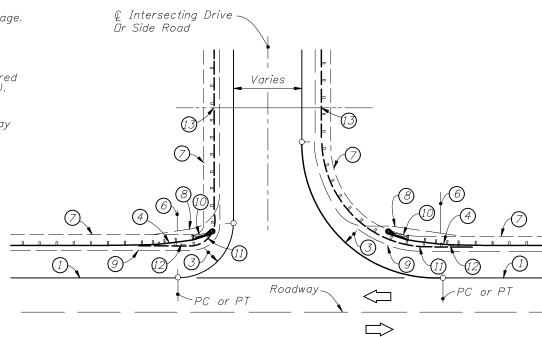
PC or PT

Dr Side Road

- $\bigcirc$  Pavement return (radius  $R_1$ ).
- Flared end anchorage to be installed except when existing guardrail on intersecting drive or side road adjoins the project.
- (5) Post for locating flare, proximate to PC or PT:
  No. 2 post for Radii 25' or less.
  No. 3 post for Radii > 25' and < 50'.
  Between No. 4 and No. 5 posts for Radii 50' or greater.
- 6 Post for locating flare, proximate to PC or PT:
  No. 3 post for Radii 25' or less.
  Between No. 4 and No. 5 posts for Radii greater than 25'.
- DExpanded shoulder for guardrail.
- (8) Expanded shoulder for flared guardrailend anchorage.
- Shoulder in absence of guardrail.
- $\bigcirc{0}$  Flared end anchorage assembly.
- (1) Radial guardrail to be installed when guardrail required on the intersecting drive or side road (radius  $R_2$ ).
- ② End anchorage Type II (radial return only).
- (3) Guardrail installation limited to roadway right of way unless otherwise called for in the plans.



Note: Only 25' and 40' radius panels are to be used for return guardrail on normal turnouts. On skewed turnouts the number of panels used and their arrangement with straight panels will be as shown in the plans or as directed by the Engineer.



TAPER TURNOUTS

Roadway

 $\triangleleft$ 

 $\Box >$ 

SIMPLE CURVE TURNOUTS

Note: The guardrail application shown on this sheet are for highways with flush shoulders and no restraints for constructing flared end anchorages and minimum lengths of guardrail. For highways with flush shoulders and restraints to constructing flared anchorages, see General Note No. 6.

Where openings in guardrail are required in close proximity to bridge traffic rails or ends of concrete barrier walls, and minimum length guardrail with flared end anchorages can not be applied, either controlled release returns or energy absorbing terminals are to be applied.

GUARDRAIL APPLICATIONS FOR INTERSECTING DRIVES AND SIDE ROADS ON RURAL FACILITIES



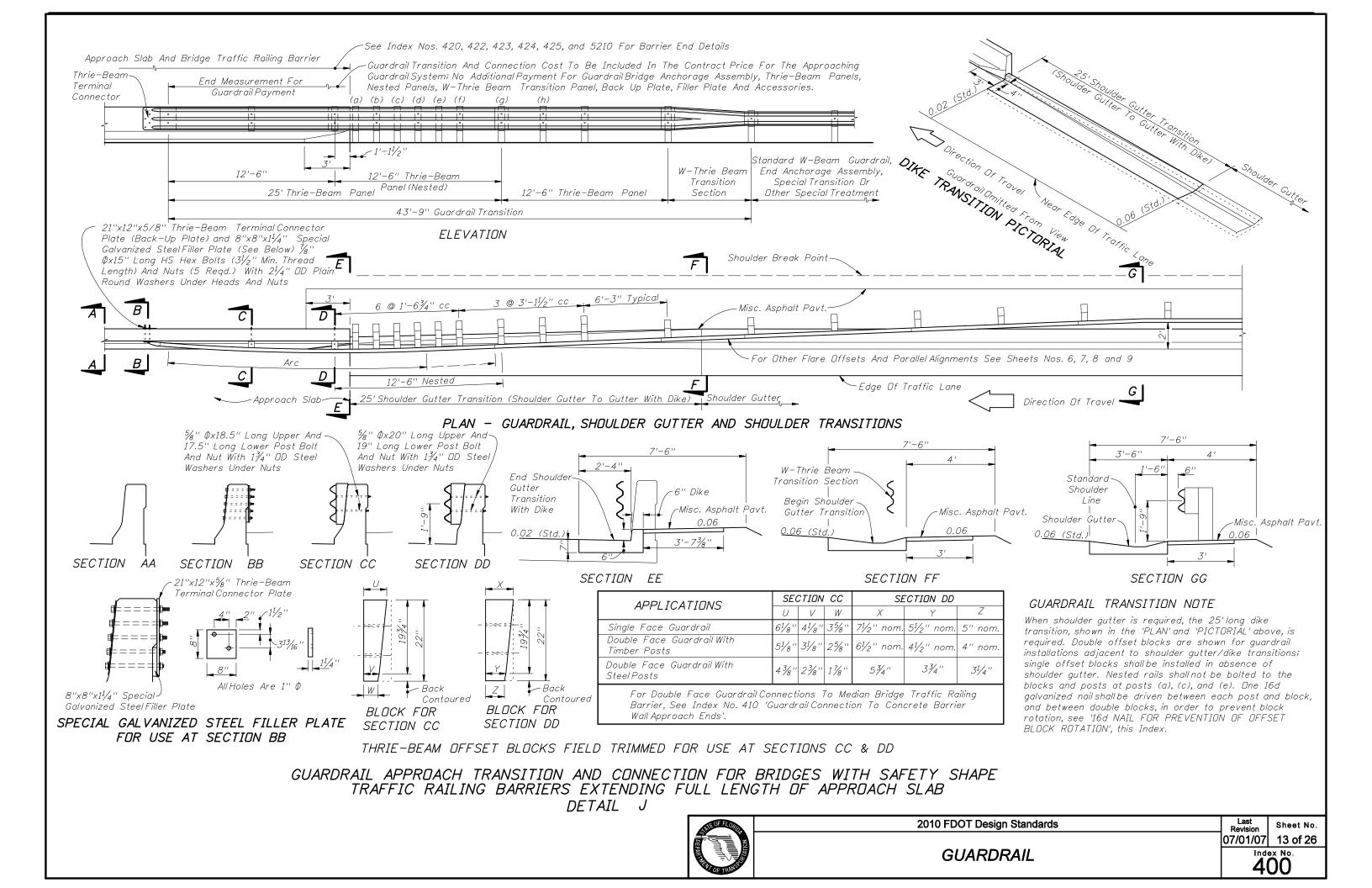
2010 FDOT Design Standards

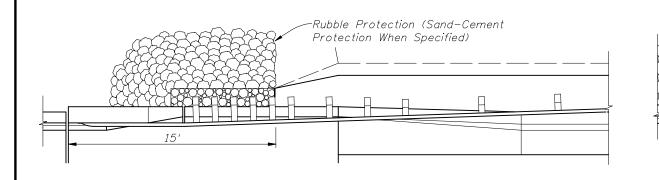
Revision 04

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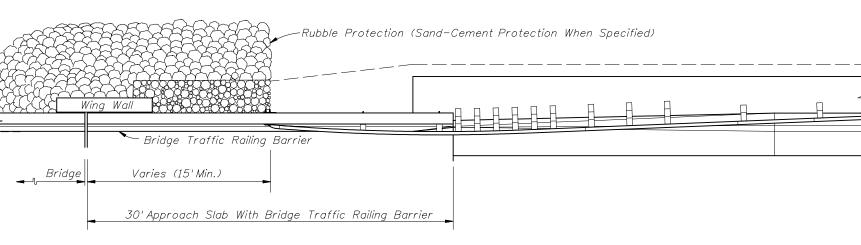
**GUARDRAIL** 

400 No.

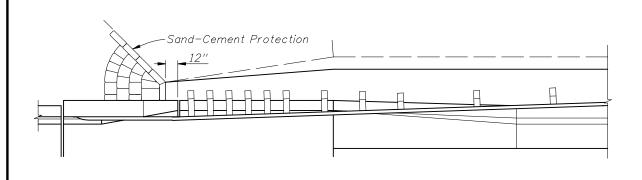




BRIDGES OVER STREAMS



BRIDGES OVER STREAMS



BRIDGES OVER RAILROADS

Limit Of Slope Pavement At Roadways
And Sand Cement Protection At Railroads

1'-6" R (Min.)

Bridge Traffic Railing Barrier

Bridge 30' Approach Slab With Bridge Traffic Railing Barrier

BRIDGES OVER ROADWAYS OR RAILROADS

For Additional Information See Index No. 402

SKETCHES - BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING LESS THAN FULL APPROACH SLAB LENGTH

For Additional Guardrail Information See Sheet 13

# SKETCHES - BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING FULL APPROACH SLAB LENGTH

#### SKETCH NOTES

- 1. These sketches are for showing shoulder interface between roadways and bridges where crossings are normal to other roadways, railroads and streams. For site specific applications and details see the plans and the FDDT Structures Design Office "Detailing Manual" and "Design Guidelines".
- 2. Shoulder treatments shown in these sketches are for locations with shoulder gutter; shoulder hinge location will vary for facilities without shoulder gutter.

#### SHOULDER INTERFACE BETWEEN ROADWAYS AND BRIDGES



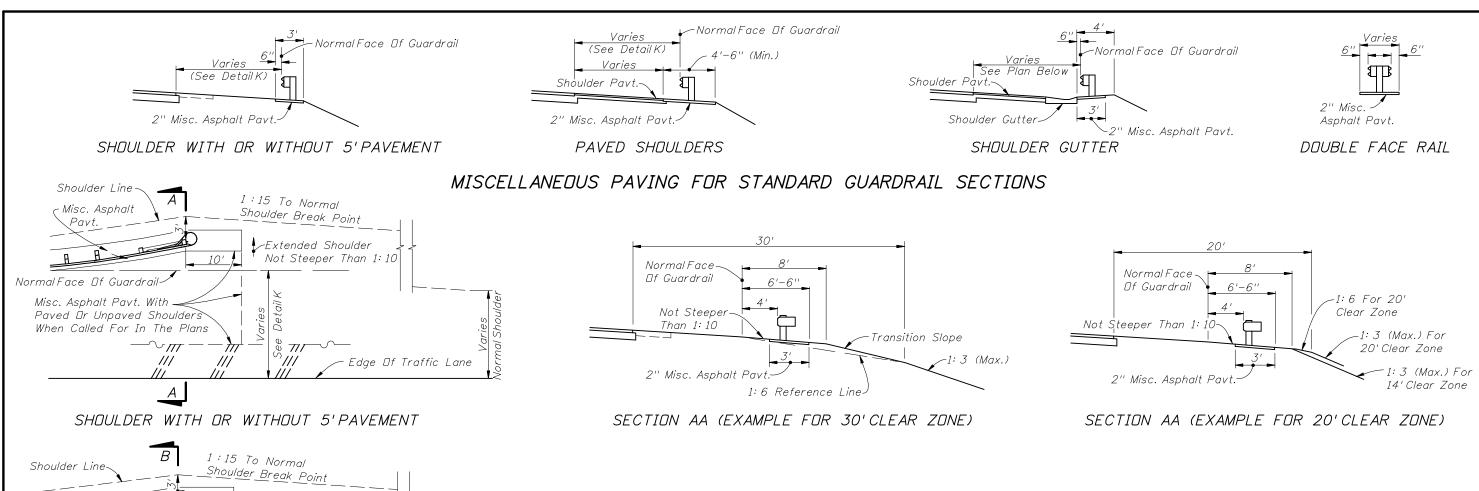
2010 FDOT Design Standards

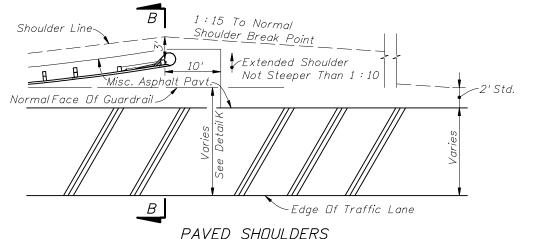
Revision 04

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**GUARDRAIL** 

400.





1:15 To Normal

SHOULDER GUTTER

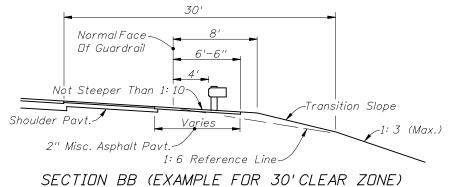
<u>Shoulder Break Point</u>

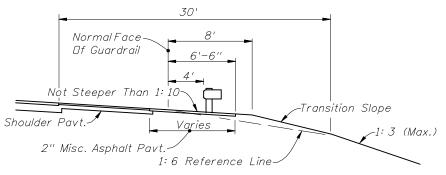
Extended Shoulder On 0.06 Rise

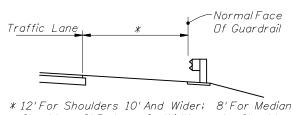
Edge Of Traffic Lane

Shoulder Line

Shoulder Gutter-



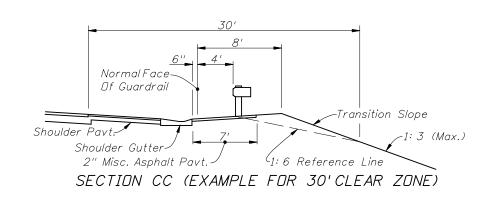




Shoulders 8' Or Less In Width; and, Shoulder Width Plus 2' For All Others Shoulders.

STANDARD LOCATIONS For Guardrail on slopes see Sheet 26.

# GUARDRAIL LOCATION-DETAIL K

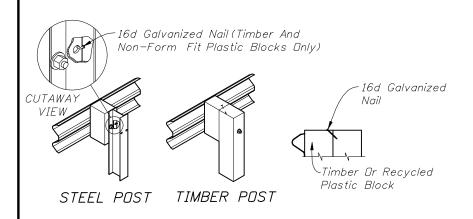


\*\* 8'For 6'Shoulders 10'For 8'Shoulders 12'For 10' And 12' Shoulders Applies To Left And Right Side Shoulders. (See Index No. 525 For Shoulder Widths And Shoulder Gutter Locations On Ramps And Auxiliary Lanes)

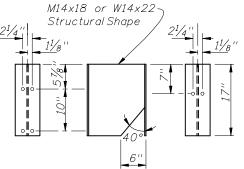
SHOULDERS, SLOPES AND MISCELLANEOUS PAVING FOR FLARED END ANCHORAGE ASSEMBLIES

-Normal Face Of Guardrail



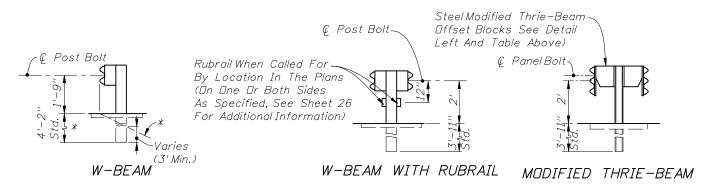


16d NAIL FOR PREVENTION OF OFFSET BLOCK ROTATION

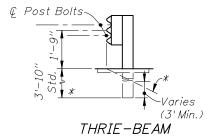


POST TRAFFIC FACE SIDE VIEW FACE

STEEL MODIFIED THRIE-BEAM OFFSET BLOCK

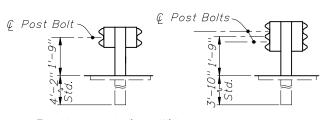


MOUNTING HEIGHTS ON SHOULDERS AND IN MEDIANS



\*Front Slope When Right Of Way, Environmental Or Other Restrictions Prohibit Normal Shoulder Extension

SINGLE FACED GUARDRAIL



For Narrow Medians With No Median Swale. See Sheet 26 For Median With Swale.

W-BEAM THRIE-BEAM

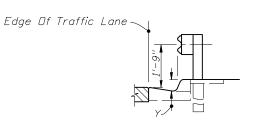
DOUBLE FACED GUARDRAIL

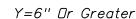
POSTS	OFFSET BLOCKS	REMARKS
Timber	Timber 6"x8"x14" (Nominal) For W-Beam And 6"x8"x22" (Nominal) For Thrie-Beam Recycled Plastic (See Notes)	Post bolt hole in timber and plastic blocks to be centered $\pm$ $^{1}\!/_{4}$ "). All timber offset blocks shall be dressed on all four sides (S4S). One 16d galvanized nail per block is to be used to prevent rotation of block (see detail left).
Steel W6x8.5, W6x9	Timber 6"x8"x14" (Nominal) For W-Beam And 6"x8"x22" (Nominal) For Thrie-Beam Recycled Plastic (See Notes)	Same as above for timber and plastic blocks except that form fit plastic block holes align with holes in steel posts and do not require nails.
Steel W6x8.5, W6x9	W14x22x17" (M14x18x17") (Steel Modified Thrie-Beam)	$\frac{5}{8}$ " $0$ x $1\frac{1}{2}$ " long hex head bolts with full length thread and nuts (2 Reqd.) and $\frac{5}{8}$ " plain round washers (4 Reqd.) for mounting steel block to post. Bolts are to be installed in opposite holes, top and bottom.

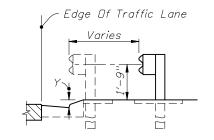
Notes: 1. Timber and recycled plastic offset blocks of identical size and shape can be intermixed within a run of rail.

2. Recycled plastic offset blocks shall meet the passing evaluation criteria for Test Level 3 of NCHRP 350. The blocks shall be tested as a component in a semi-rigid guardrail test article under full scale crash test conditions. The blocks shall be in conformance with Sections 536 and 972 of the Specifications and be included on the Qualified Products List. W-Beam blocks shall be 14" in height and thrie-beam blocks shall be 22" in height. The blocks shall be capable of providing a 7½" (Min.) offset.

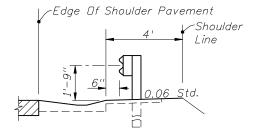
# PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS







Y=Less Than 6"



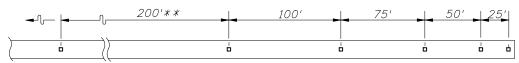
Shoulder Gutter

NOTE

For location of guardrail with offset behind curb and gutter refer to the Plans Preparation Manual, Volume 1, Section 4.3.5.

LOCATION AT CURB & GUTTER SECTIONS-DETAIL L

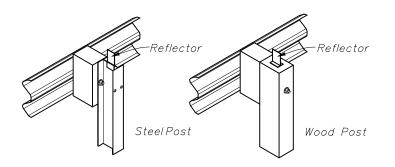




Note: Adjustment in spacing may be required to fit exact quardraillengths as directed by the Engineer. For minimum installations (length 62.5') provide one reflector at each end and one at the approximate center.

\*\*For curves greater than 2° the spacing shall be reduced to 100' through the curve.

#### REFLECTOR SPACING

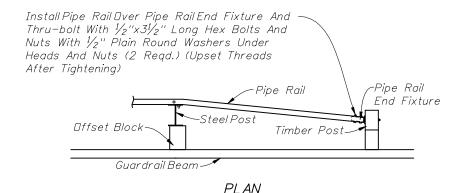


# PICTORIAL VIEW REFLECTOR MOUNTING

# REFLECTORS-DETAIL M

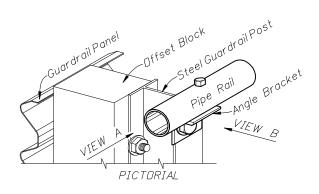
#### REFLECTOR NOTES

- 1. Reflectors shall conform to Section 993 of the Standard Specifications.
- 2. Reflector color (white or yellow) shall conform to the color of the near lane edgeline.
- 3. Reflectors installed on median guardrail shall have retroreflective sheeting on both sides of the reflector.
- 4. The cost for reflectors shall be included in the contract unit price for Guardrail.



Steel Pipe

Attach Pipe Rail End Fixture To Post With  $\frac{1}{2}$ "x7" Long Hex Bolts And Nuts With  $\frac{1}{2}$ " Plain Round Washers Under Heads And Nuts (2 Regd.) (Upset Threads After Tightening) -This Post Can Be Timber Only Steel Post-Guardrail Bean Pipe Rail Γορ Of Curb. Lip Of Gutter— Pipe Rail End Fixture Gutter-



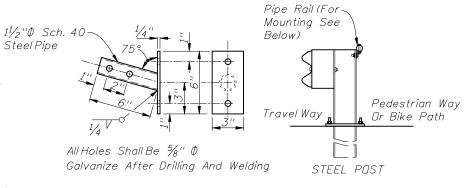
#### ELEVATION

# NOTES

1. Pipe Rail required on steel guardrail posts when pedestrian ways and bikeways are located 4' or less from back of the posts. Pipe rail shall not extend beyond the last post of the approach end anchorage assemblies. Begin and end the pipe rail in accordance with the Pipe Rail End detail.

Refer to Sheet 1, Note 6 for guardrail end treatment requirements.

- 2. When guardrails with timber posts are located with the back of posts 4' or less from the near edge of the pedestrian way or bikeway, the bolt ends will require one of the following treatments:
- (a) Trimming back flush with the face of nut and metalizing or
- (b) Use of post bolts 15" in length with the washers and nuts counter sunk into sinks 1" to  $1\frac{1}{2}$ " deep or
- (c) Use of post bolts 15" in length with sleeve nuts and washers.
- 3. The cost for Pipe Rail, mounting components and installation shall be included in the contract unit price for guardrail. Bolt end treatment for timber post shall be included in the contract unit price for guardrail.



PIPE RAIL END FIXTURE

SECTION

 $\frac{5}{8}$ " $\phi$  Bracket And Pipe Holes With  $\frac{1}{2}$ "x $\frac{3}{2}$ " Long Hex Bolt And Nut With  $\frac{1}{2}$ " Plain Round Washer (Upset Threads After Tightening) 2" Nom. Ø Sch. 40 Galv. Pipe Rail ¾"♥ Bracket Hole With %"x2 Long Hex Bolt And Nut With 5/8' Plain Round Washers (Upset Threads After Tightening) Steel Guardrail Post -Steel Guardrail Post 2½"x2"x½"x4" -11/8" Offset From Long Angle Bracket © Of Guardrail Post (Galvanized) VIEW A VIEW B

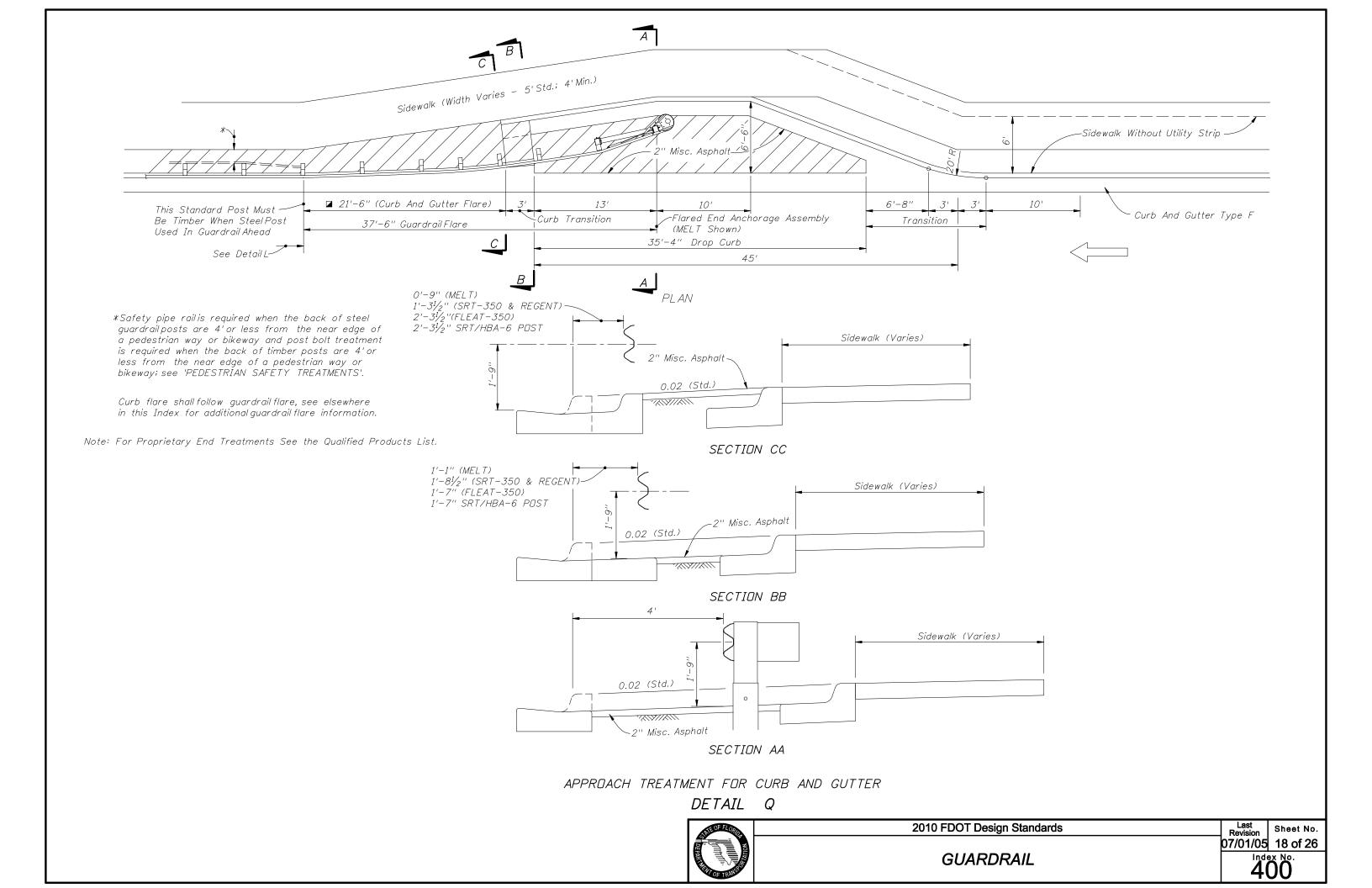
FOR LOCATIONS USED BY PEDESTRIANS OR CYCLISTS PEDESTRIAN SAFETY TREATMENTS

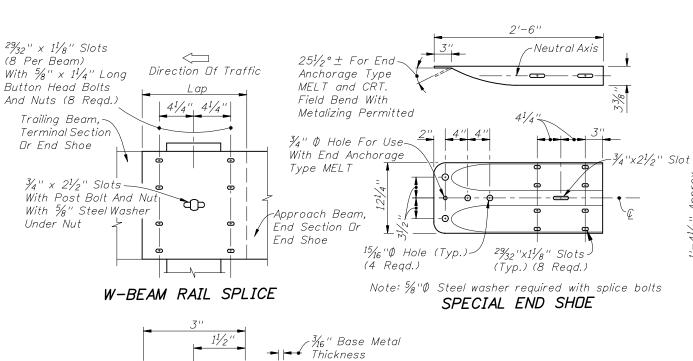
2010 FDOT Design Standards

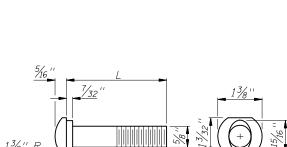
Sheet No. 07/01/09 17 of 26

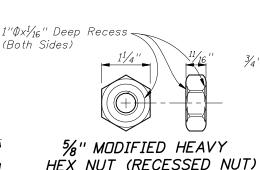
PIPE RAIL MOUNTING

400







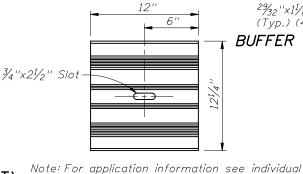


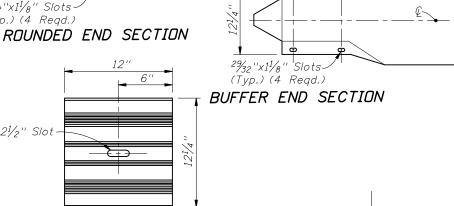
 $2'-3\frac{1}{2}'$ 

<sup>29</sup>/<sub>32</sub>''x1<sup>1</sup>/<sub>8</sub>'' Slots

(Typ.) (8 Reqd.)

FLARED END SECTION





<sup>2</sup>9/<sub>32</sub>''x1<sup>1</sup>/<sub>8</sub>'' Slo

Sheet-

Thickness

31/4'

W-BEAM

⊕

1'-3" R Standard

 $(10^{1}/_{2}" R When Used$ 

For End Anchorage

Type MELT)

81/2"

Varies

Contour To Fit

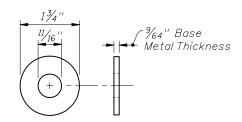
Over Beam

Note:

For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary to accomplish alignment or where the posts bolt head shows tendency to pull through the rail slot. Washers installed on guardrail, between end anchorages, prior to July 1, 1990 may remain in place until the quardrail is relocated or until repairs require removal and reinstallment of a post bolt.

11/16"x1" Slot

# (RECTANGULAR PLATE WASHER) BEAM WASHER



Note:

The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by  $\frac{5}{8}$ "  $\phi$  hex bolts and nuts and under hex nut for connecting rubrail to wood and steel posts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

THREAD LENGTH *APPLICATION* (In.) (Min.) (In. 11/4" Rail Splice Bolt Full Length Single Or Double Faced Guardrail Post Bolt - Timber Or Recycled Plastic Offset 10" Block(s) On Steel Post As An Option, A Single 25"\* Long Post Bolt May Be Used 18" 4" Post Bolt - Single Faced Guardrail Timber Posts Post Bolt - Double Faced Guardrail Timber Posts 25"\* Double Faced Guardrail Steel Posts

Special bolts having lengths of 10" or greater shall have a thread length of not less than 4". For applications where special bolts having lengths greater than 25" are required, the Contractor may use a  $\frac{5}{8}$ " $\emptyset$ threaded rod (field cut to length). A hex nut and beam washer shall be used at the guardrail face with no more than  $rac{3}{4}$ " of the threaded rod projecting beyond the top of the nut. The projecting thread on both ends shall be distorted to secure the nuts, and both ends of the threaded rod metalized with organic zinc-rich coating. \*Use of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced quardrail that results in the bolt projecting more than 34" beyond the face of the nut after pull-up shall be trimmed to  $\frac{3}{4}$ " reveal and metalized with organic zinc-rich coating.

5%" OVAL SHOULDER BUTTON HEAD BOLT

# W-BEAM BACK-UP PLATE

end anchorage assembly details.

<b>OFFSETS (Ft.)</b> Measured From Face Of Guardrail To Front Of Above Ground Rigid Hazard					
POST SPACING (Ft.)		E BEAM Thrie-Beam		D BEAMS Thrie-Beam	
6'-3''	4′	3'-3''	N/A	N/A	
3'-11/2''	3'	2'-8''	2'-8''	2'-4"	
1'-63/4''	N/A	N/A	2'-4"	2'	

Contour To Fit

Over Beam

<sup>29</sup>/<sub>32</sub>"x1<sup>1</sup>/<sub>8</sub>" Slots

(Typ.) (4 Regd.)

The values shown should be utilized unless changes are supported by imperical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5.4 of the AASHTO Roadside Design Guide are cautioned to proceed only if background in the table development is understood.

## MINIMUM OFFSET FOR SINGLE FACED GUARDRAIL (Ft.)

HS Hex bolts for THRIE-BEAM TERMINAL CONNECTORS shall conform to the requirements of ASTM A449 (Type 1) with heavy hex nuts and washers. All other hex bolts shall conform to the requirements of ASTM A563. Bolts, nuts and washers shall be hot dip galvanized. Heavy hex nut may be used in lieu of hex nuts and hex nuts used for jam nuts.

HEX BOLTS AND NUTS



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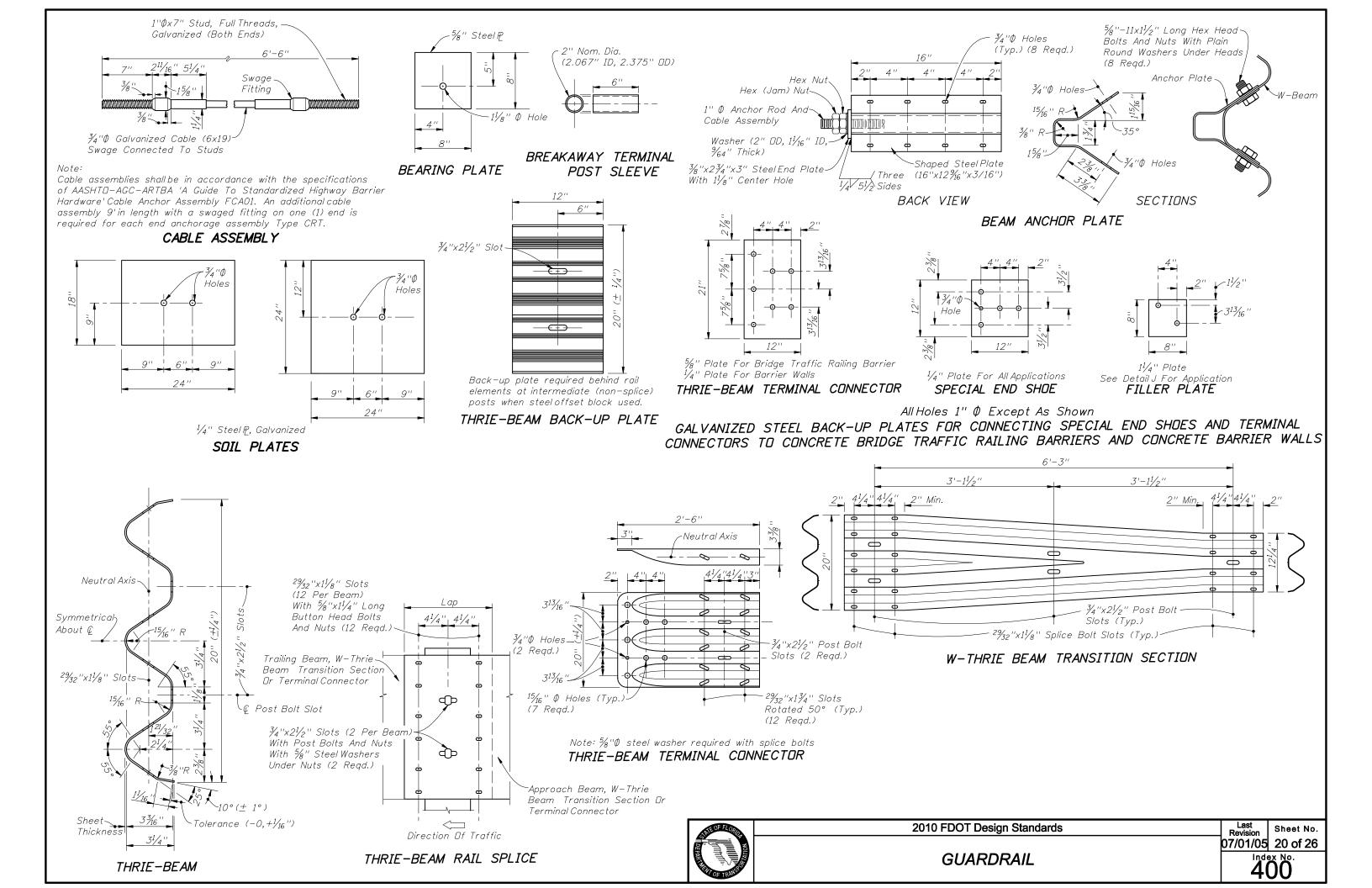
-Tolerance

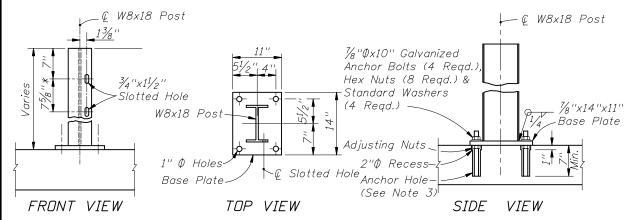
 $(-0, + \frac{1}{16}'')$ 

5%" STEEL WASHER

**GUARDRAIL** 

400°



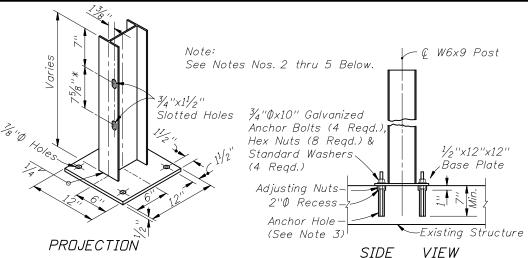


## FOR REPLACEMENT OF EXISTING W8x18 GUARDRAIL POSTS ON APPROACH SLABS AND BRIDGES

\* Additional slotted hole required when mounting thrie-beam guardrail

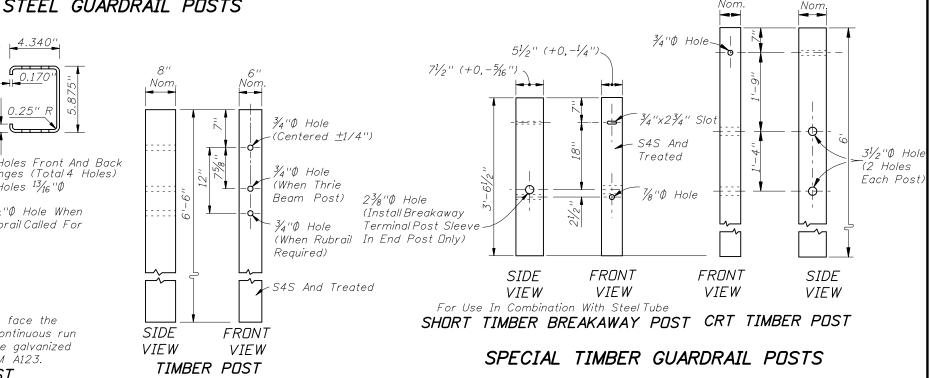
NOTES: (SPECIAL STEEL POST)

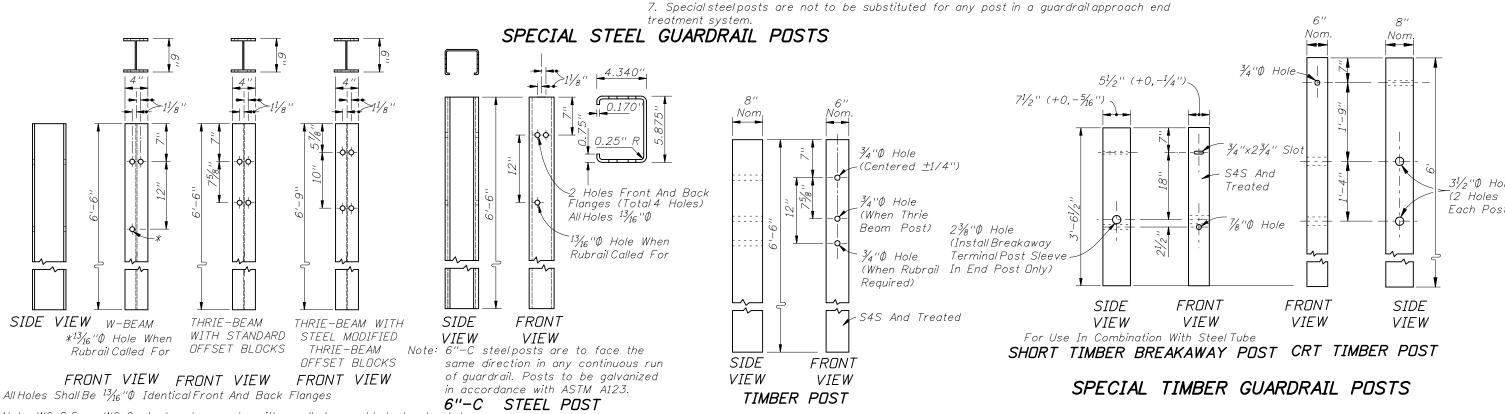
- 1. See Index No. 402 for special steel posts required for construction and repair of quardrail transitions to bridge traffic railing barrier retrofits on existing bridges. See Structures Index Nos. 470 through 476 for steel posts required to construct traffic railing barrier retrofits on existing bridges.
- 2. Either anchor bolts, concrete wedge anchors or approved Adhesive-Bonded Anchors for Structural Applications may be used. Anchor bolts, wedge anchors and adhesive anchors shall have a minimum tensile strength of 60,000 psi and galvanized in accordance with ASTM A153 (stainless steel components may be substituted but components plated in accordance with ASTM B-633 are not acceptable). Adhesive anchor rods shall be equal in diameter to that detailed for anchor bolts. Wedge anchors are to be installed in accordance with the manufacturer's recommendations, assuming 3,000 psi compressive strength for concrete. Wedge anchors shall also meet the following requirements:
- (a) tensile load each anchor: approach slabs 14,000 lbs.; other structures 8,000 lbs. (b) shear load each anchor: approach slabs 15,000 lbs.; other structurers 7,800 lbs.



# FOR CONSTRUCTION OF GUARDRAIL WHERE CULVERT, PIER FOOTING OR OTHER STRUCTURE PRECLUDES DRIVEN POST INSTALLATION

- 3. Posts are to be plumbed by adjusting nuts or mortar seating. Posts installed using anchor bolts and adhesive anchors are to be set with adjusting nuts as detailed, unless the Engineer approves the use of mortar seating in lieu of adjusting nuts. Posts installed using wedge anchors are to be set with mortar seating. Base plates shall be grouted with neat finish.
- 4. Adhesive-Bonded Anchors for Structural Applications shall comply with Section 937 and be installed in accordance with Section 416. Drilled hole diameter shall be in accordance with the manufacturer's instructions.
- 5. Anchor holes and recesses shall be drilled; wedge anchor holes are to be drilled in accordance with the manufacturer's specifications. Encountered reinforcing steel shall be drilled through. Holes shall be thoroughly cleaned when setting bolts and anchors and dry when setting wedge anchors.
- 6. Steel post and base units shall be galvanized in accordance with ASTM A123. Any damaged galvanized areas are to be metalized in accordance with Section 562 of the Standard Specifications.





Note: W6x8.5 or W6x9 steel posts may be either rolled or welded structural shapes conforming to or exceeding the design properties of ASTM A6/A6M. Welding shall be in accordance with the requirements of ASTM A769/A769M. Posts shall be cut to length and the ends sealwelded between web and flange before galvanizing. Posts to be galvanized in accordance with ASTM A123

W6x8.5 OR W6x9 STEEL POST STANDARD TIMBER AND STEEL GUARDRAIL POSTS 2010 FDOT Design Standards

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6" V

(6'– Pari

SIDE VIEW FRONT VIEW

For Use In Combination With

Short Timber Breakaway Post

STEEL TUBE

**GUARDRAIL** 

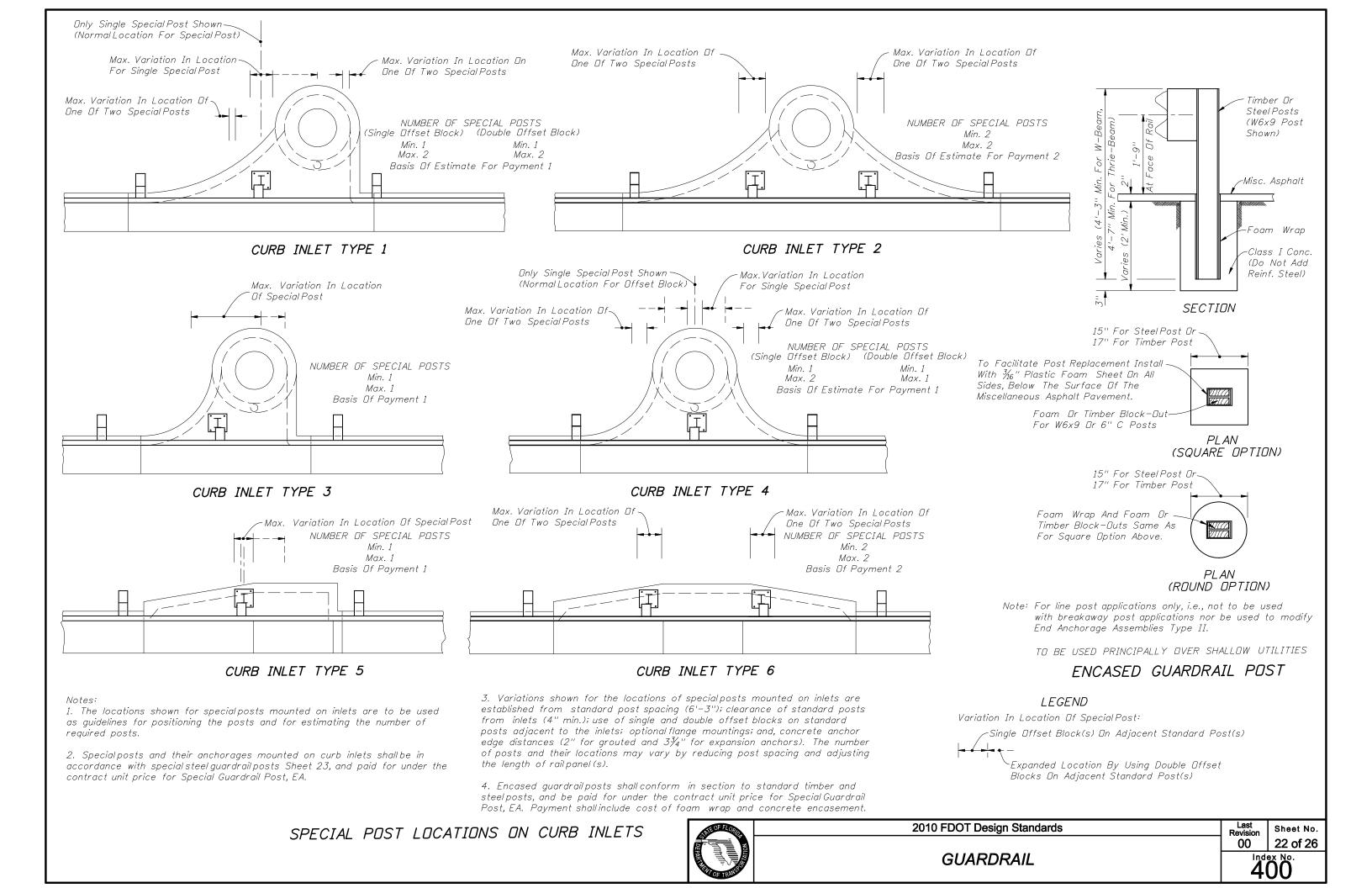
3/4" Holes-

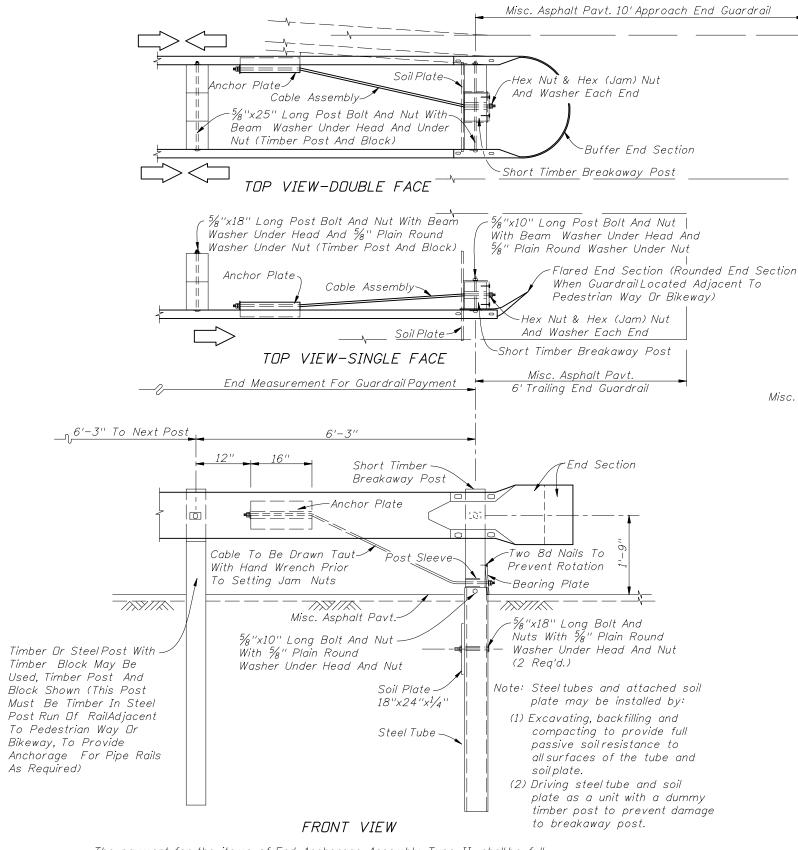
TS 8"x6"x3/6

Open End-

Galvanized

400

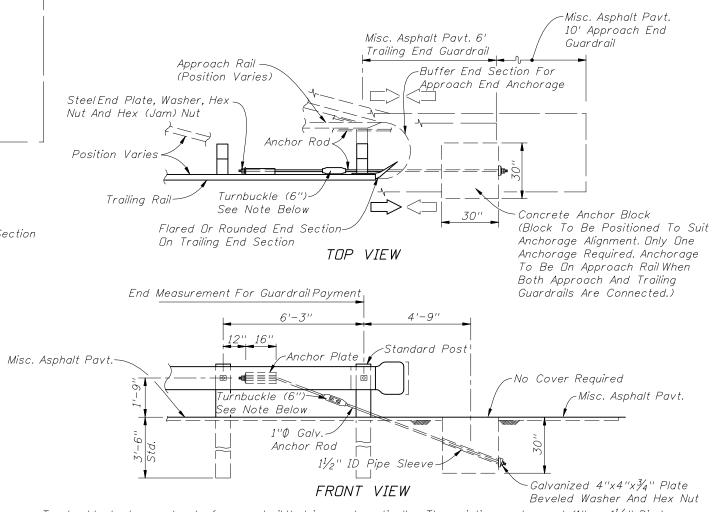




The payment for the items of End Anchorage Assembly Type II shall be full compensation for furnishing and installing either the Round or the Buffer End Section, the Beam Anchor Plate, Cable Assembly, Pipe Sleeve, Soil Plate, Steel Tube, Bearing Plate, Short Timber Breakaway Post, Offset Blocks and the necessary hardware.

CABLE ANCHOR OPTION

END ANCHORAGE ASSEMBLY TYPE II



Turnbuckle to be used only for guardrail that is reset vertically. The existing anchor rod (1" or  $1\frac{1}{4}$ " Dia.) shall be field cut, threaded 4" on each end, and, metalized in accordance with Sections 562 and 975 of the Standard Specifications. The cost for cutting, threading, metalizing and the turnbuckle shall be included in the contract unit price for Reset Guardrail, LF.

The payment for the items of End Anchorage Assembly Type II shall be full compensation for furnishing and installing the Beam Anchor Plate, Anchor Rod, Pipe Sleeve, Anchor Block, either Flared, Rounded or Buffer End Section, and the necessary hardware.

#### CONCRETE ANCHOR BLOCK OPTION

#### TYPE II NOTES

- 1. Unless specified in the plans, the contractor can supply either the cable anchor option or the concrete anchor block option.
- 2. Type II end anchorage assemblies are approved for all speeds and are intended for use as: (a) trailing end anchorages for single face free standing guardrail systems;
  - (b) approach end anchorages for single face free standing guardrail systems when end anchorage is located outside of the clear zone; and,
  - (c) both approach and trailing ends of double face guardrail systems.

Crash cushions shall be constructed at or in lieu of approach Type II end anchorages located inside the clear zone.

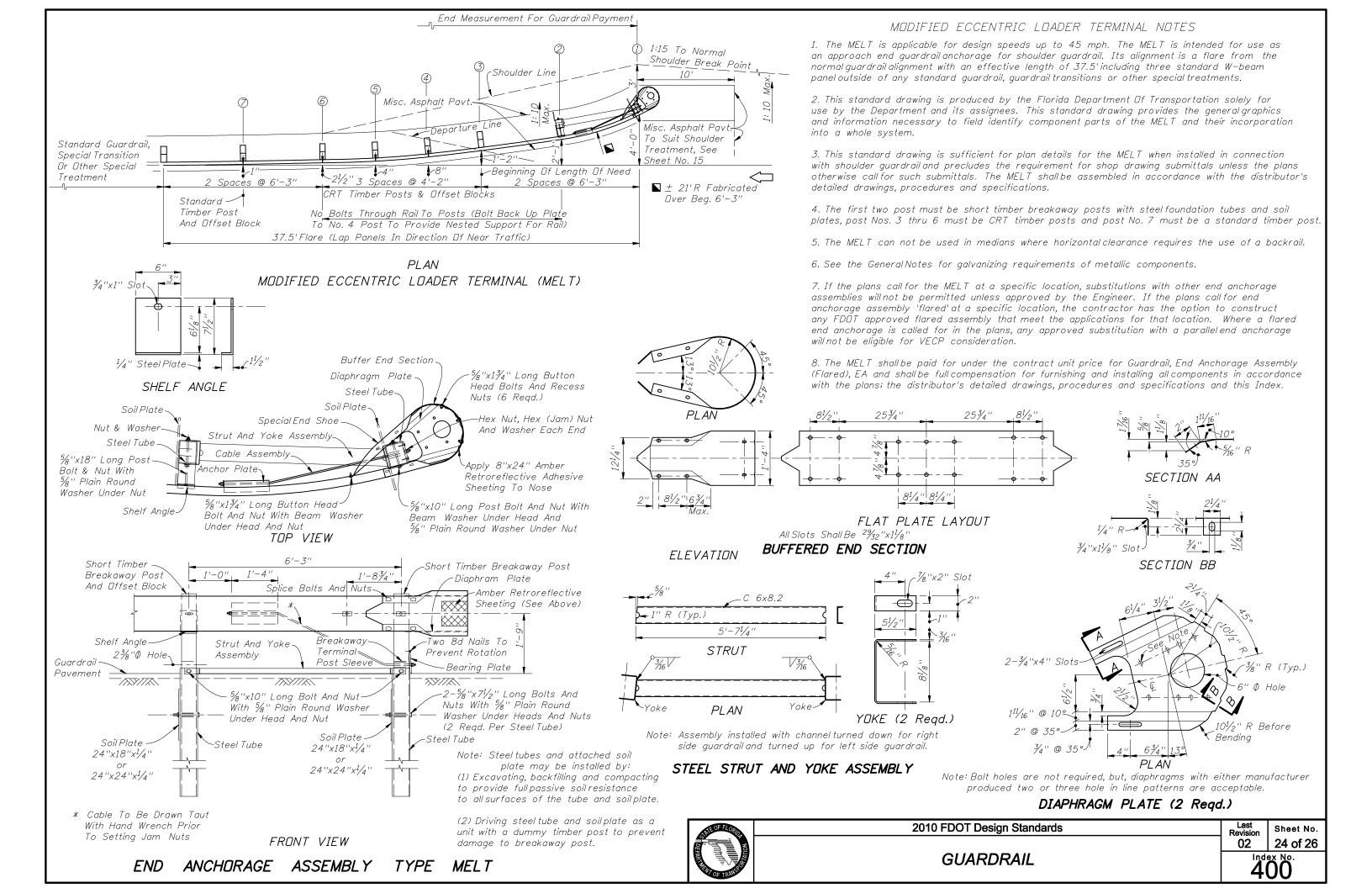
End anchorage for thrie beam guardrail shall be constructed the same as detailed for W-beam, except use thrie beam rail and end section; and the Anchor Plate is to be attached to the bottom corrugation of the thrie beam.

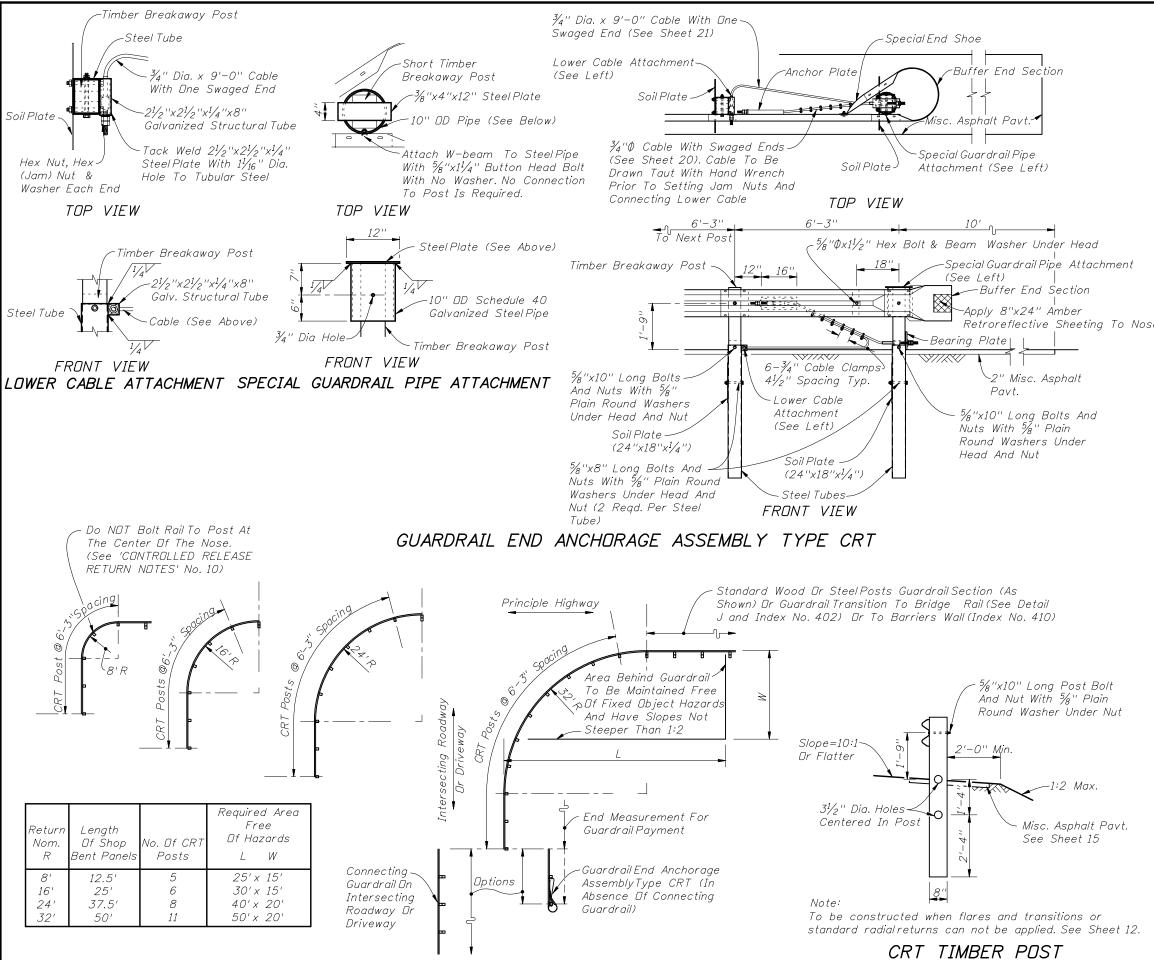
3. These end anchors are to be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Type II), EA as called for in the plans or by permit.



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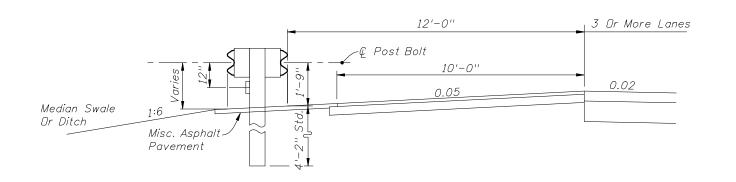
CONTROLLED RELEASE RETURN FOR SIDE ROAD AND DRIVEWAY ACCESS

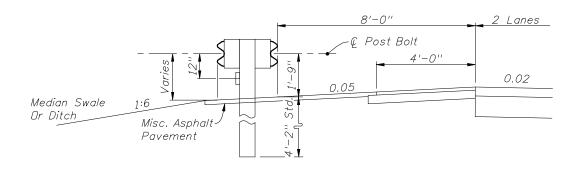
#### CONTROLLED RELEASE RETURN NOTES

- 1. Controlled release returns are intended for use (a) in openings in continuous quardrail for driveway and side road access when flares and transitions or standard radial returns can not be applied (Sheet 12); and, (b) for shielding the ends of bridge traffic rails and barrier walls where the driveway and side road access is in close proximity to the structure and space does not permit the proper use of approved flared and parallel types of Guardrail End Anchorage Assemblies.
- 2. Controlled release returns are not intended as a substitute or replacement for the appropriate use of approved vehicle impact attenuators.
- 3. Controlled release returns with either 8', 16' or 24' radii are designed for highway speeds of 60 mph or less; the 32 radius return is to be used only highway speeds of 60 mph or less; the 32' radius return is to be used only for highway speeds of 45 mph or less.
- 4. The controlled release returns shown are designed as full returns based on an intersection angle of 90°. Retroreflective Sheeting To Nose The return can be terminated with the Guardrail End Anchorage Assembly Type CRT or connected to standard guardrail as shown or as otherwise detailed
  - 5. The Guardrail End Anchorage Assembly Type CRT is to be used only for the controlled release returns with 8', 16', 24' and 32' radii as shown; the assembly is not to be used in any tangent rail or flared rail applications. Other types of end anchorage assemblies are not to be used in the controlled release returns.
  - 6. The area immediately behind the control release return shall have slopes not steeper than 1:2 and be maintained free of fixed objects in accordance with the area limits tabulated in the plan below.
  - 7. The surface approaching the controlled release return shall have a transverse slope not exceeding 1:10. The effective width of the transverse surface is to be based on standard vehicle departure, return radii and preceding shielding; the width (beyond shoulder) shall be not greater than the corresponding 15' and 20', 'W' values tabulated
  - 8. The curved guardrail portion of the controlled release return shall be full section shop bent panels (12.5' or 25' panels).
  - 9. Washers are not to be used between the guardrail beam and the head of the button head post bolts at any controlled release terminal (CRT) post or at any Guardrail End Anchorage Assembly Type CRT breakaway timber post.
  - 10. The guardrail beam of the 8' radius return is not bolted to the center control release post.
  - 11. See the General Notes for galvanizing requirements of metallic components.
  - 12. Controlled release return systems shall be paid for under the contract unit prices for Guardrail (Roadway), LF, Guardrail (Shop-bent Panels), LF, and Guardrail, End Anchorage Assembly (Type CRT), EA as called for in the plans or by permit and shall be full compensation for furnishing and installing all components in accordance with the plans and with this index. CRT posts are included in the cost for guardrail.

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#### Notes:

- 1. Typical placement shown. May be constructed at other locations as called for in the plans.
- 2. Rubrail required on median side or ditch side of barrier.

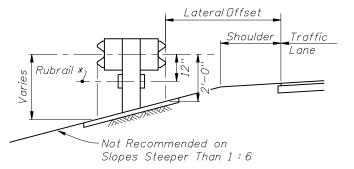
MOUNTING HEIGHT FOR DOUBLE FACED GUARDRAIL ON MEDIAN SHOULDERS (FREEWAYS)

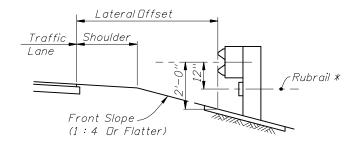
# LATERAL PLACEMENT ON SLOPES (FROM EDGE OF NEAR TRAFFIC LANE)

Slope	Standard Guardrail <sub>(2)</sub>	Guardrail Not Recommended	Guardrail With Rubrail <sub>(3)</sub>
1:4	to 13'	14' to 27'	28' to 45'
1:5	to 14'	15' to 25'	26' to 45'
1:6	to 16'	17' to 22'	23' to 45'
1:7	to 20'	21' to 24'	25' to 45'
1:8	to 25'		26' to 45'
1:9	to 26'		27' to 45'
1:10	to 27'		28' to 45'

#### Notes:

- (1) For shoulders less than 12' in width the tabulated values will be reduced by the difference between 12' and the shoulder width. Placement of guardrail on frontslopes steeper than 1:4 not recommended. Cost of rubrail to be included in the contract unit price for guardrail.
- (2) Standard guardrail; 1'-9" to & post bolt. Rubrail required on median side when double face guardrail is used.
- (3) Guardrail with rubrail; 2'-0" to @ post bolt.



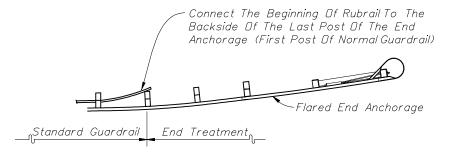


GUARDRAIL ON MEDIAN SLOPES

GUARDRAIL ON OUTSIDE SLOPES

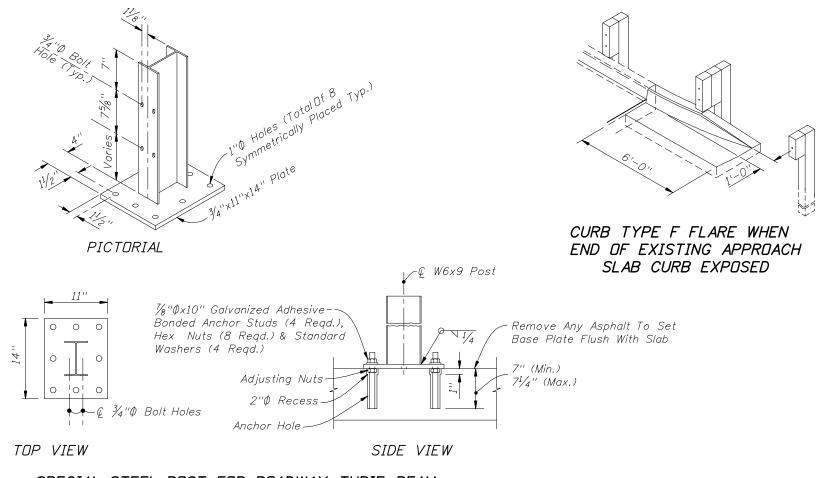
\* C6x8.2, Plates And Fastners or Bent Plate And Fastners In Accordance With Standards RLR01 And RER01 Of AASHTO-AGC-ARTBA "A Guide To Standardized Highway Barrier Hardware")

## GUARDRAIL ON SLOPES

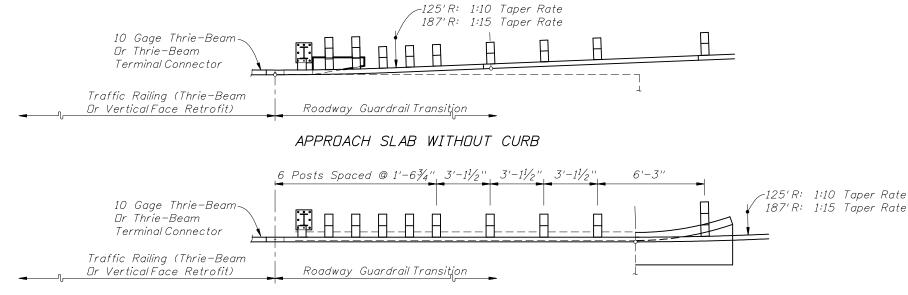


## RUBRAIL TERMINATION





# SPECIAL STEEL POST FOR ROADWAY THRIE-BEAM TRANSITIONS TO BRIDGE TRAFFIC RAILING RETROFITS

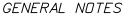


#### APPROACH SLAB WITH CURB

Longitudinal Location Of Transition Blocks And Curb End Flares Will Vary With Scheme Type

#### PARTIAL PLAN VIEWS

# GUARDRAIL TRANSITION ALIGNMENTS FOR BRIDGE THRIE-BEAM AND VERTICAL FACE TRAFFIC RAILING RETROFIT



- 1. This index provides thrie-beam transition and connection details for approach end guardrail on existing bridges, and anchorage details for trailing end traffic railing retrofits and safety shapes on existing bridges. Sheets 1 through 23 apply to bridges with retrofitted traffic railings, (Sheet 23 shows the trailing end guardrail connections). Sheet 24 applies to bridges with safety shaped traffic railing.
- 2. The schemes identified by Arabic numerals in this index are complementary to the bridge traffic railing barrier retrofit schemes with like numeral identification in Index Nos. 470, 471 through 476, 480 through 483. The schemes in this index identified by Roman numerals are complementary to bridge safety shaped traffic railing barrier where determined to be in accordance with applications of criteria specified in the Structures Manual.
- 3. For guardrail applications and details of related hardware and accessories that are not provided on this index, refer to Index No. 400.

#### NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

- 1. The transition detailshown on this sheet shows (a) the standard post spacings within the typical thrie-beam approach transitions connecting to existing bridges with retrofit traffic railings, and (b) depict the typical alignments of the approach transitions.
- 2. The curb and gutter flare shown on this sheet is typical of flares that are to be constructed when approach slab curbs extend to the beginning of the slab, and where other treatment to curb blunt ends are not in place.
- 3. The special steel post for roadway thrie-beam transitions detailed on this sheet is specific to all transition applications on this index that require one or more steel posts.

The special steel post and base plate assembly shall be fabricated using ASTM A36 or ASTM A709 Grade 36 steel. Welding shall conform to ANSI/AASHTO/AWS D1.5. The assembly shall be hot-dip zinc coated in accordance with Section 536 of the Specifications.

Anchor studs shall be fully threaded rods in accordance with ASTM F1554 Grade 36 or ASTM A193 Grade B7. All nuts shall be heavy hex in accordance with ASTM A563 or ASTM A194. Anchor studs and nuts shall be hot-dip zinc coated in accordance with the Specifications. After the nuts have been snug tightened, the anchor stud threads shall be single punch distorted immediately above the top nuts to prevent loosening of the nuts. Distorted threads shall be coated with a galvanizing compound in accordance with the Specifications.

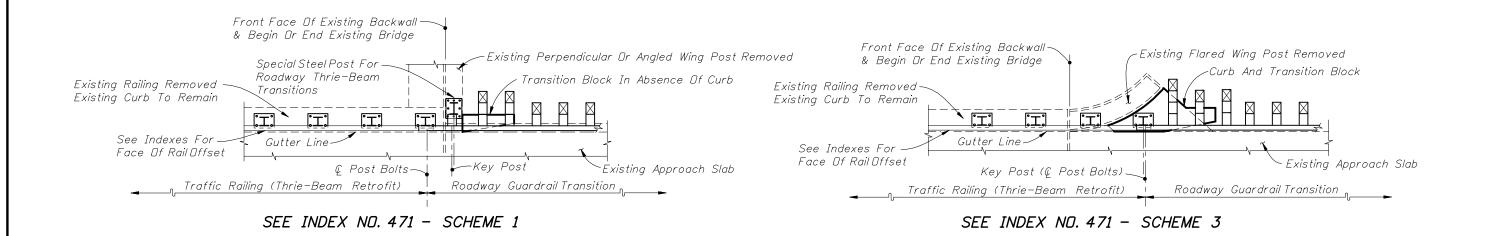
Adhesive bonding material systems for anchors shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.4. Nested beam extensions and points for terminal connector attachments will vary for traffic railing barrier vertical face retrofits. The plan views for the vertical face retrofit barriers show the primary configurations for each particular scheme. The associated pictorial views show the variations.

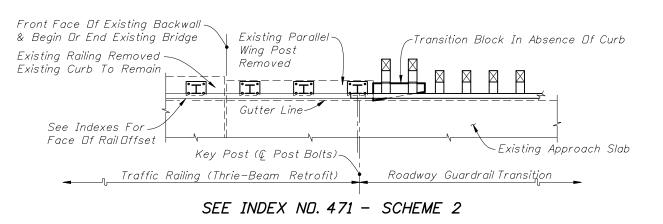
- 5. For installing thrie-beam terminal connector to traffic railing vertical face retrofits, see notations on Sheets 12 through 15 and the flag notation on Sheet 23.
- 6. Payment for connections to traffic railing vertical face retrofits are to be made under the contract unit price for Bridge Anchorage Assembly, EA., and shall be full compensation for bolt hole construction, terminal connector, terminal connector plate and bolts, nuts and washers.

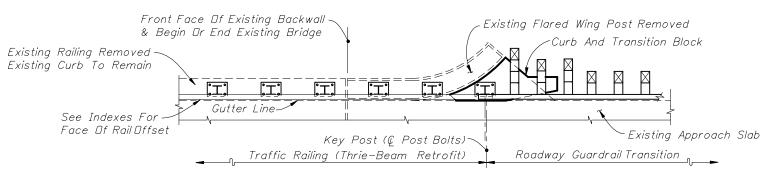
# DESIGN NOTES FOR GUARDRAIL TRANSITIONS CONNECTING TO TRAFFIC RAILING RETROFITS ON EXISTING BRIDGES

1. For selection of an appropriate transition scheme, see the Structures Manual for instructions to the Structures and Roadway engineers.



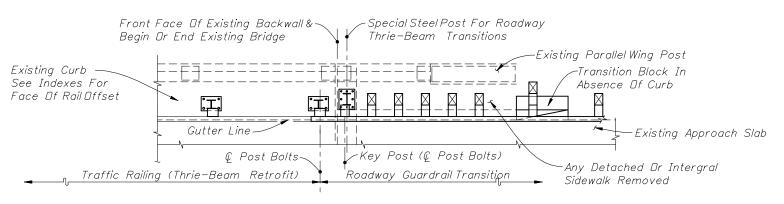




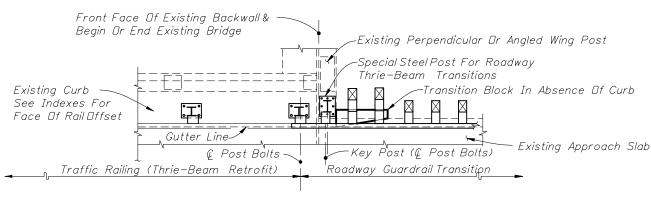


SEE INDEX NO. 471 - SCHEME 3

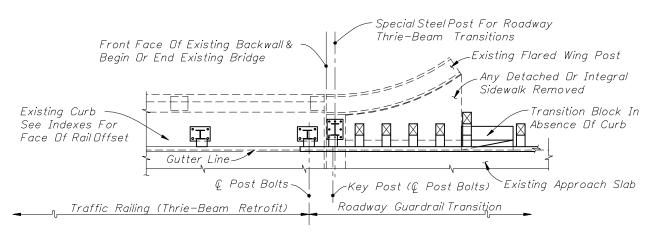




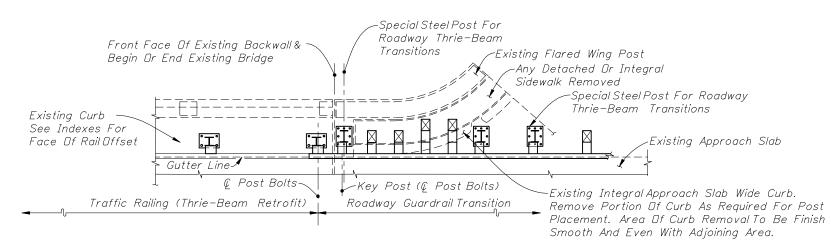
SEE INDEX NOS. 472 & 475 - SCHEME 2



SEE INDEX NOS. 472 & 475 - SCHEME 1

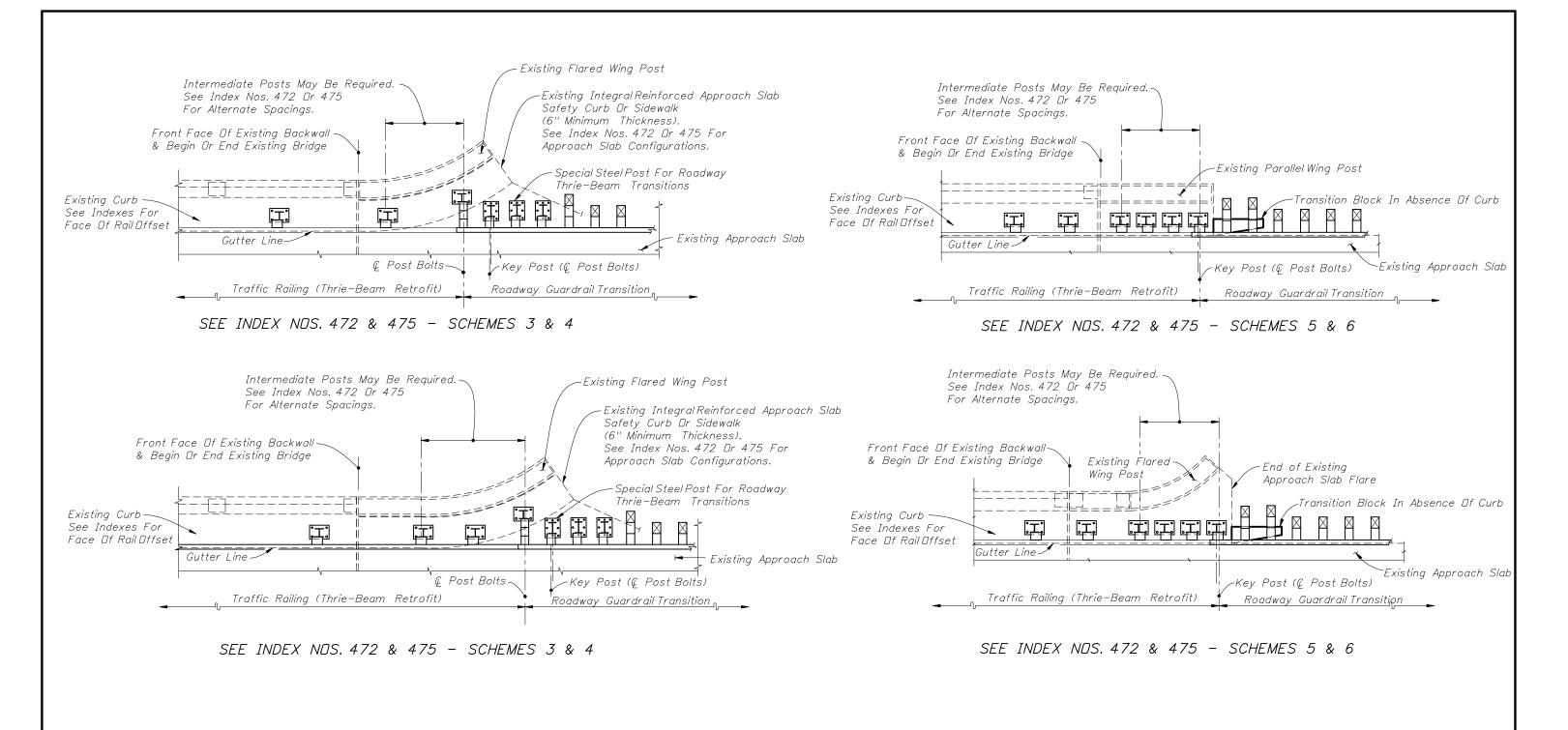


SEE INDEX NOS. 472 & 475 - SCHEME 2



SEE INDEX NOS. 472 & 475 - SCHEME 2





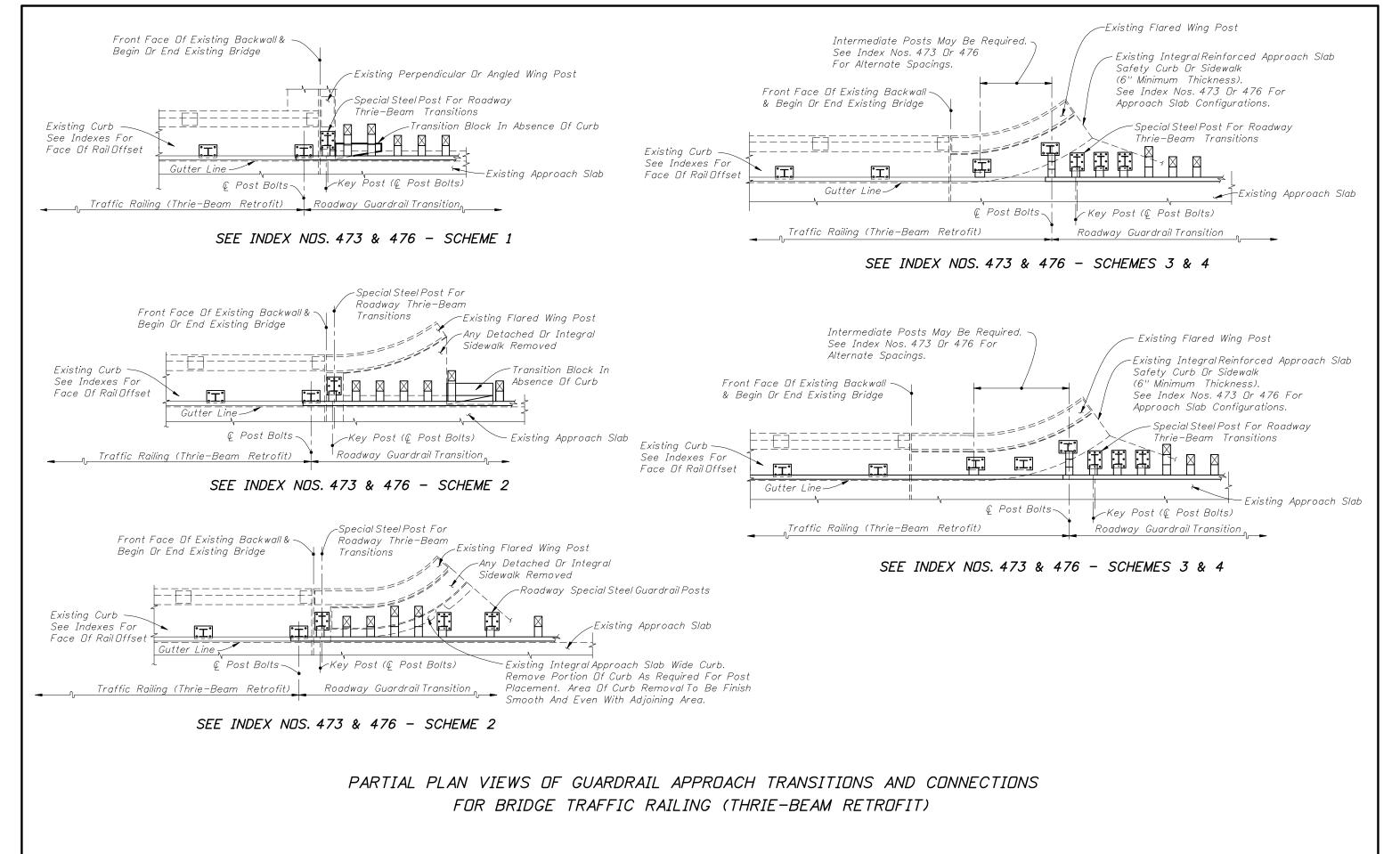


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Sheet No.

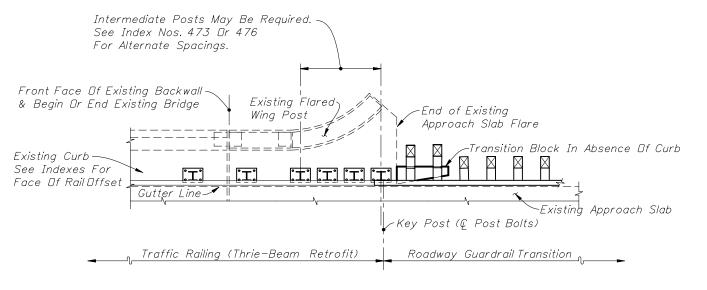


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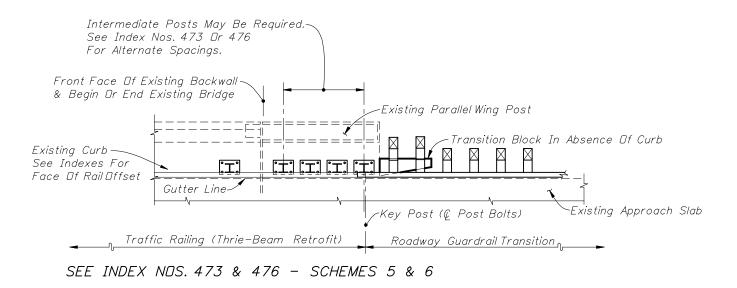
GUARDRAIL TRANSITIONS AND
CONNECTIONS FOR EXISTING BRIDGES

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Index No. 402

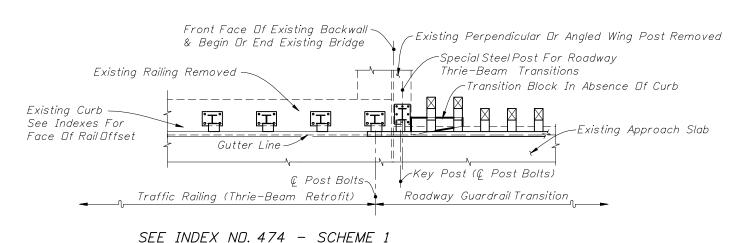


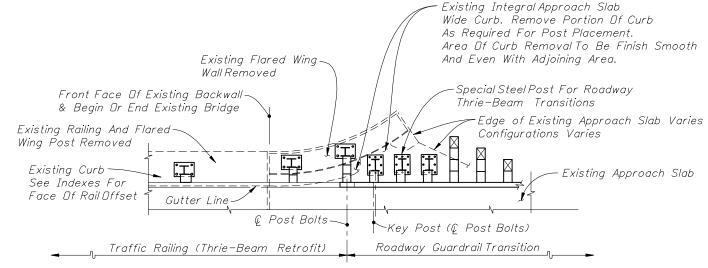
SEE INDEX NOS. 473 & 476 - SCHEMES 5 & 6



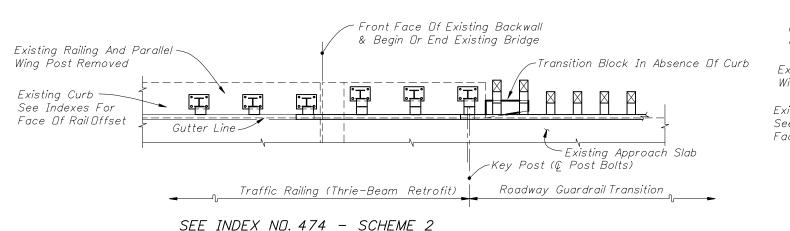
PARTIAL PLAN VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS
FOR BRIDGE TRAFFIC RAILING (THRIE-BEAM RETROFIT)

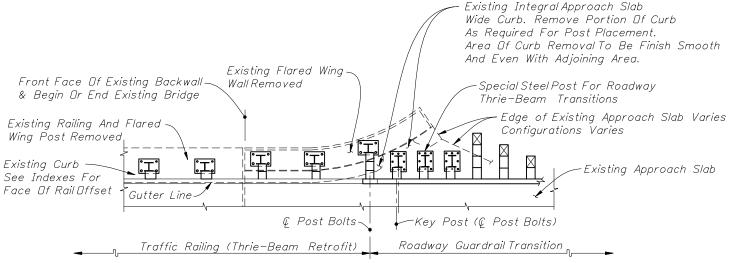






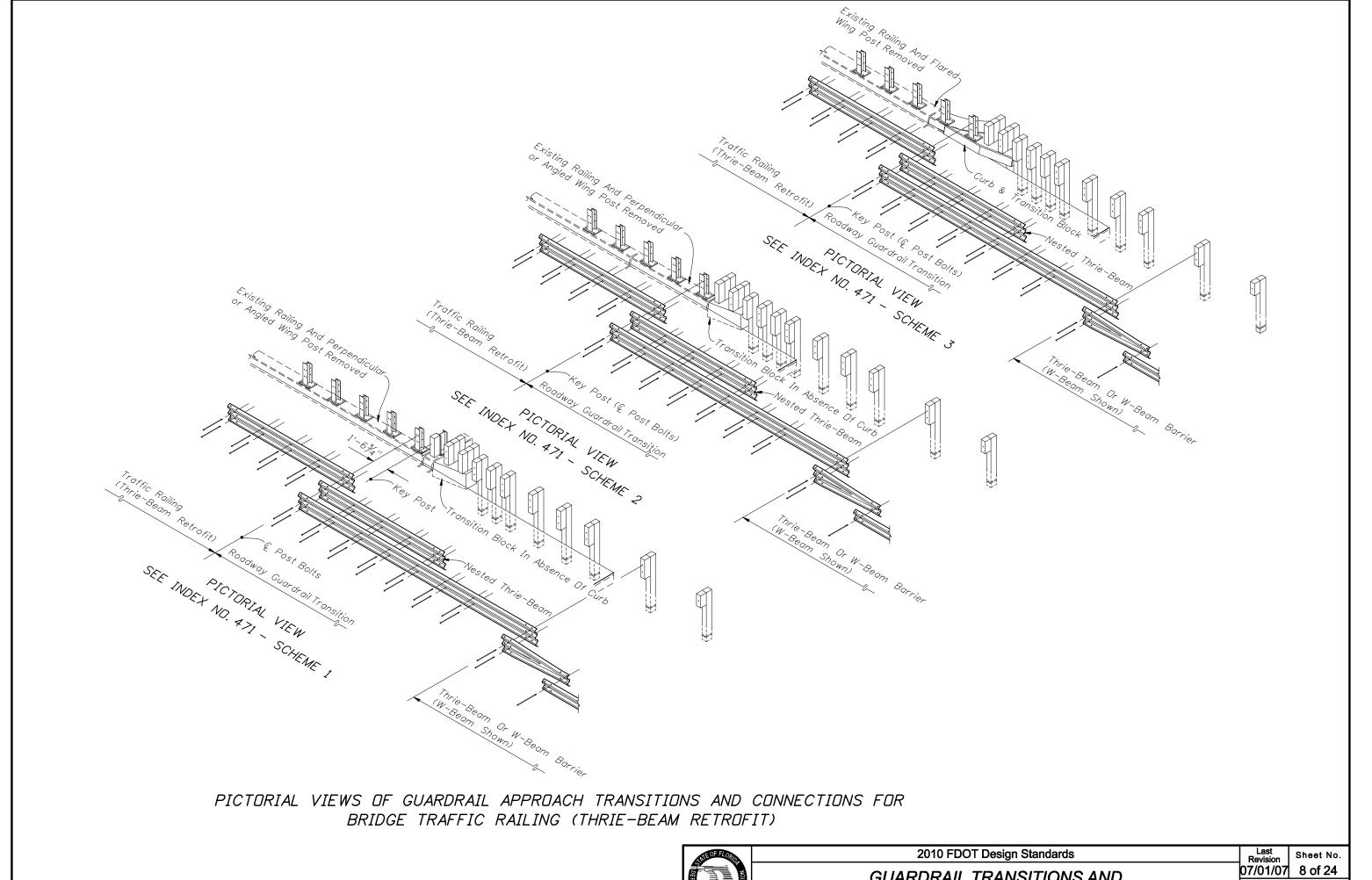
SEE INDEX NO. 474 - SCHEME 3

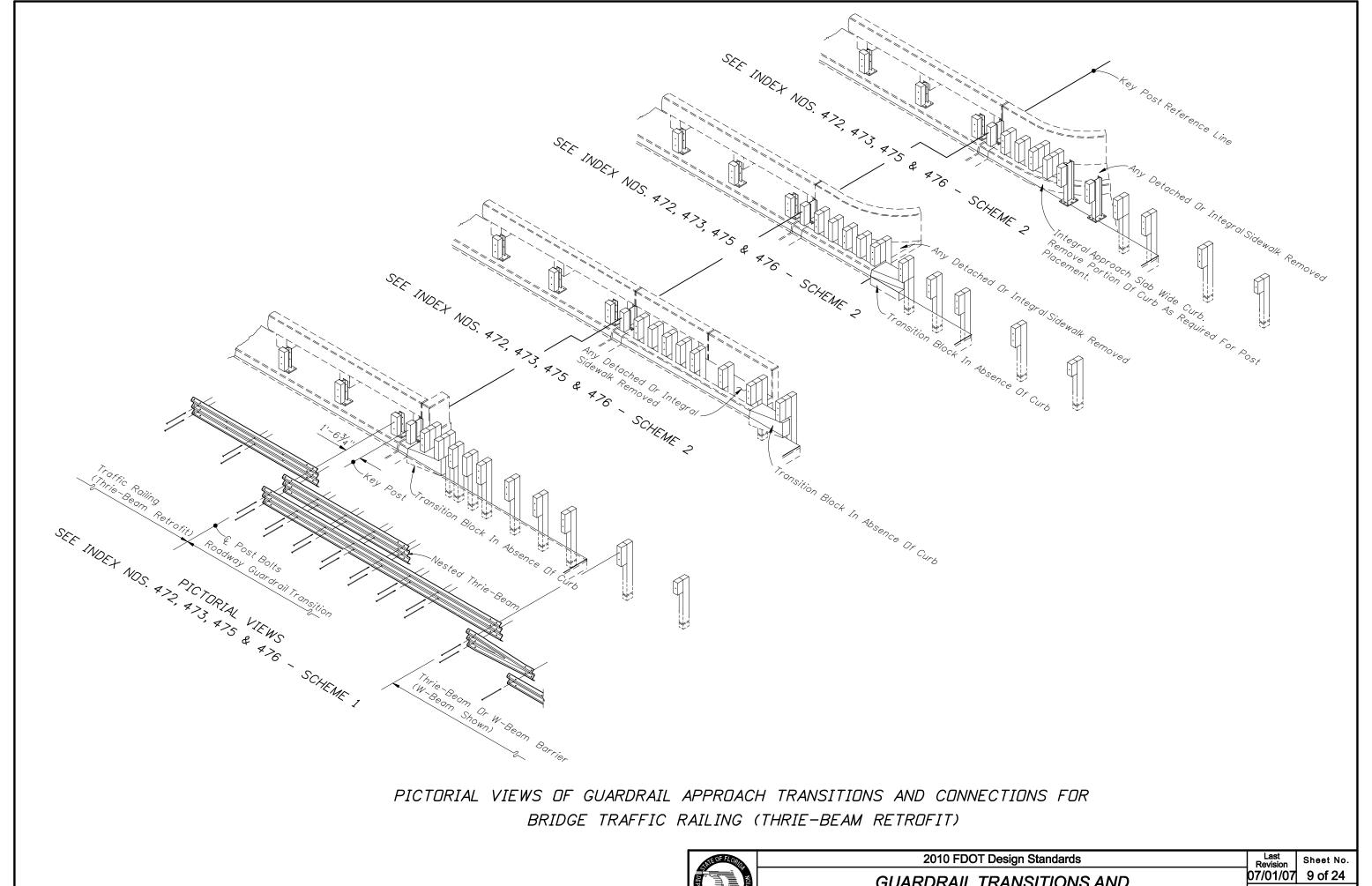




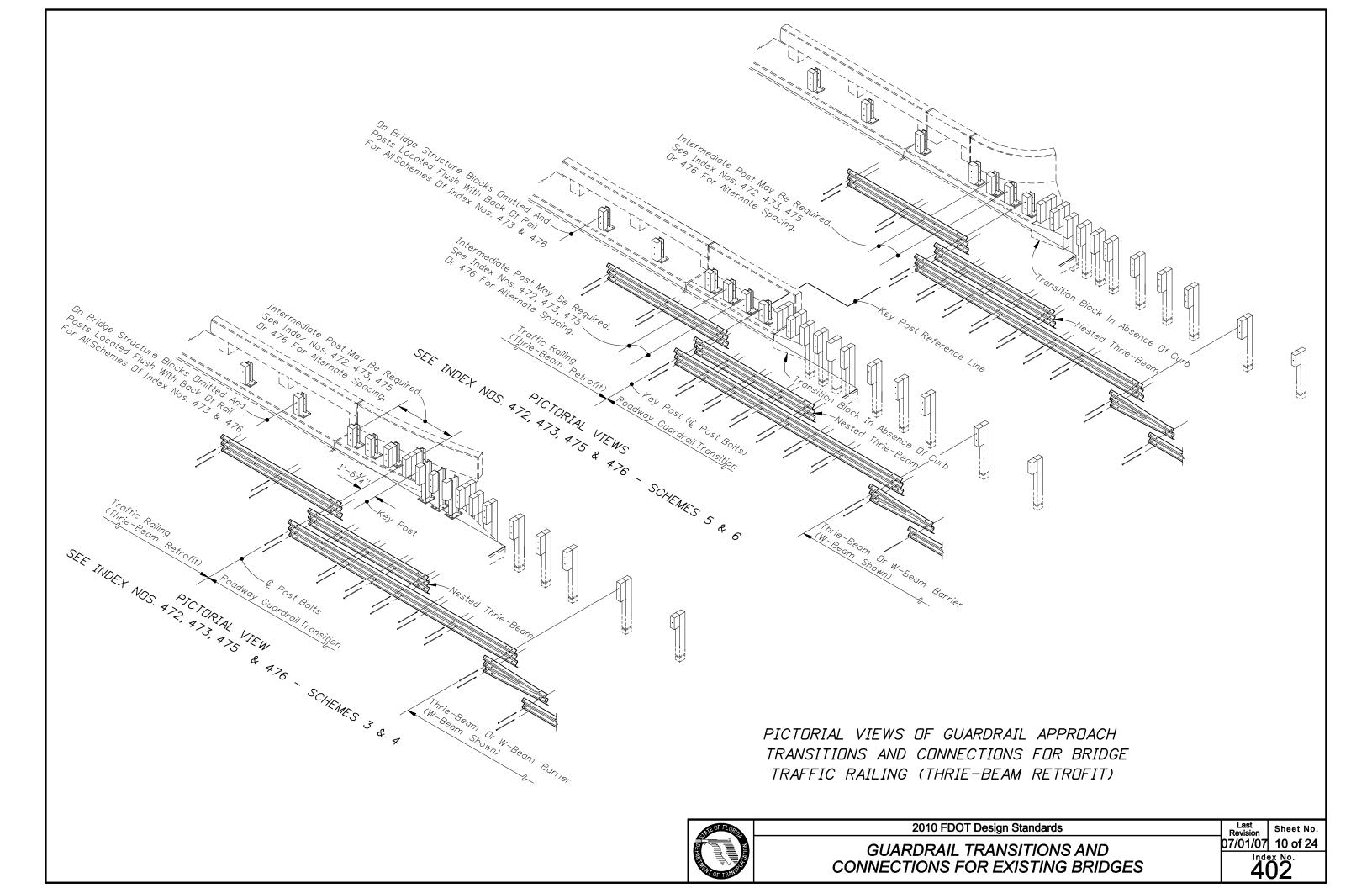
SEE INDEX NO. 474 - SCHEME 3

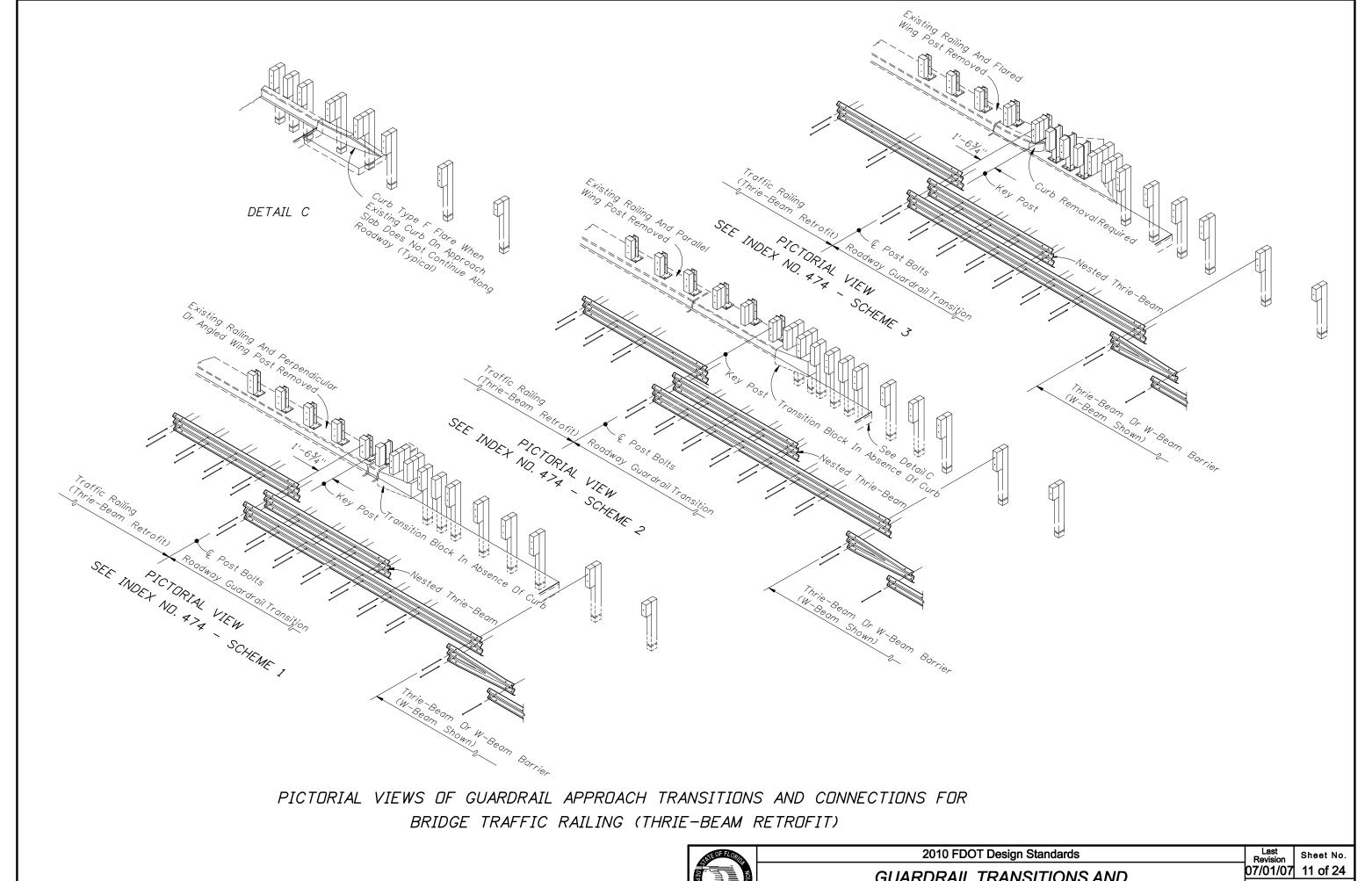


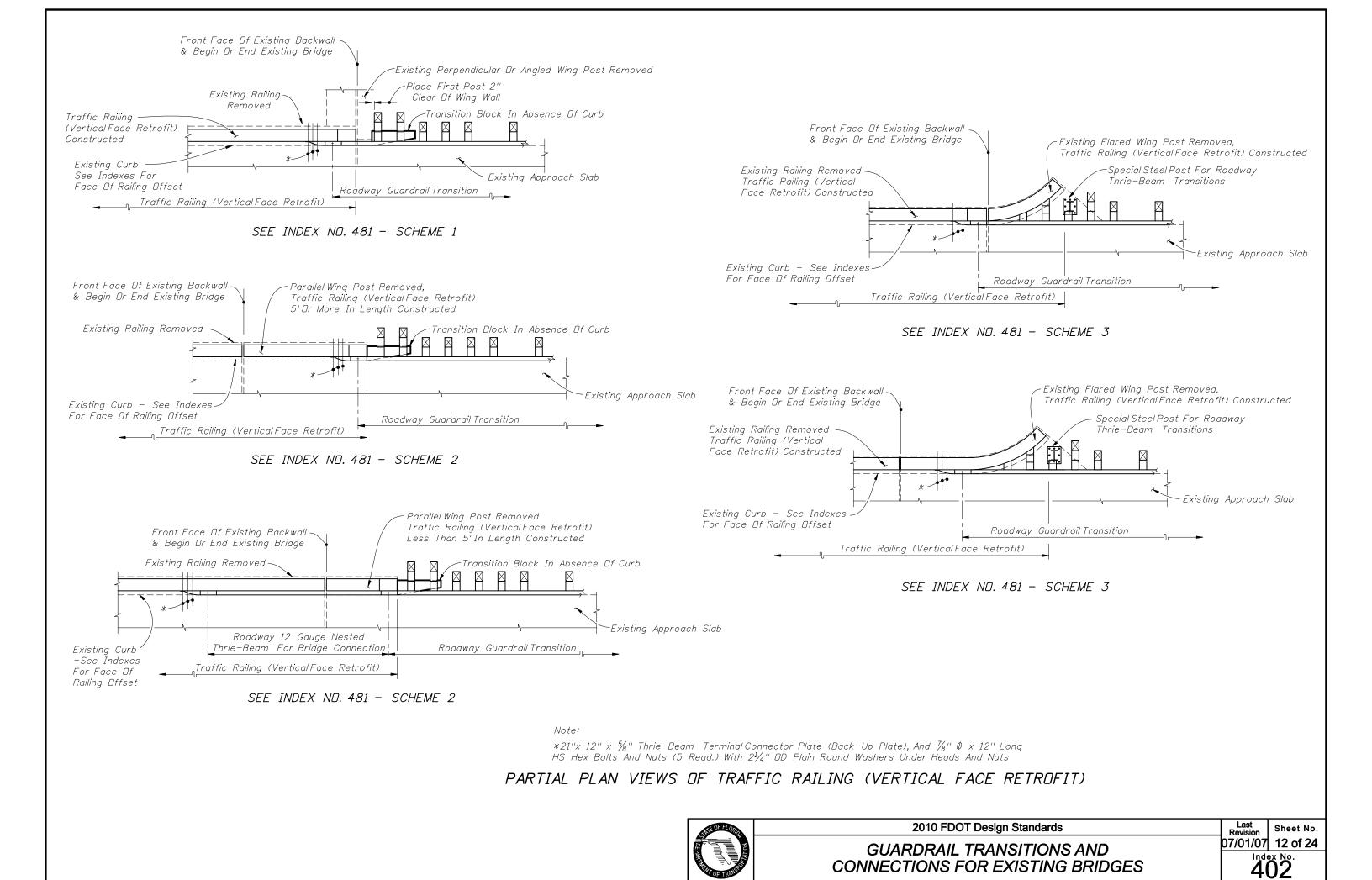


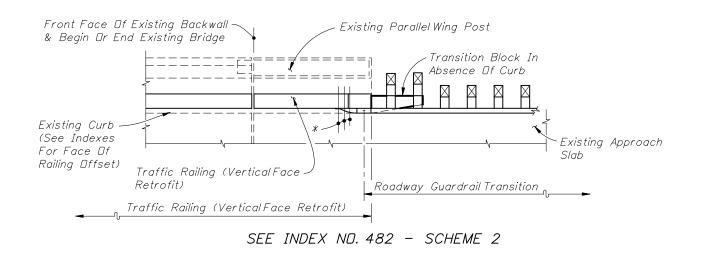


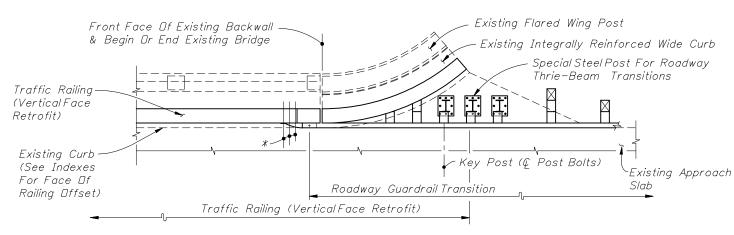
402



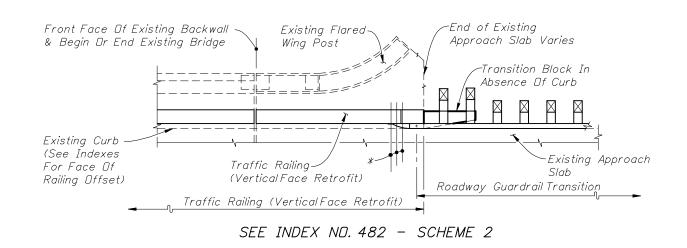


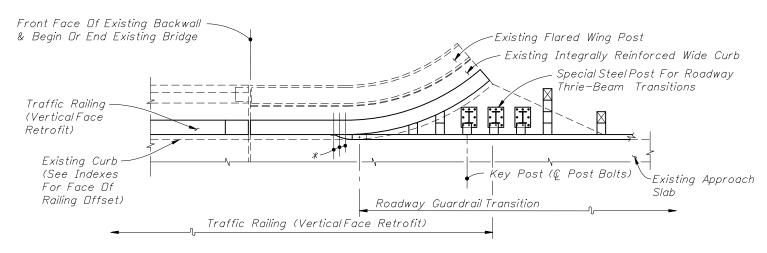






SEE INDEX NO. 482- SCHEME 3



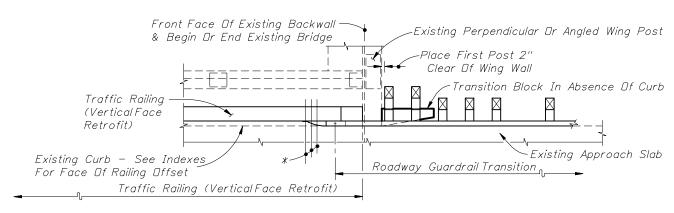


SEE INDEX NO. 482- SCHEME 3

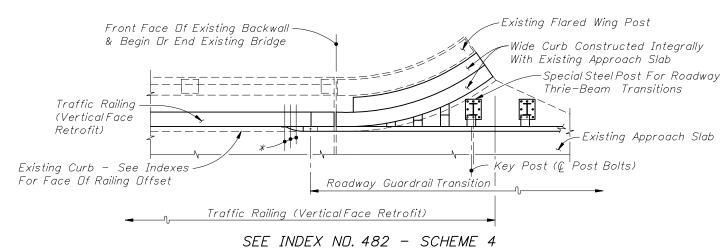
Note: \*21" x 12" x  $\frac{5}{8}$ " Thrie-Beam Terminal Connector Plate (Back-Up Plate), And  $\frac{7}{8}$ "  $\phi$  x 12" Long HS Hex Bolts And Nuts (5 Reqd.) With  $2\frac{1}{4}$ "  $\alpha$ 0D Plain Round Washers Under Heads And Nuts

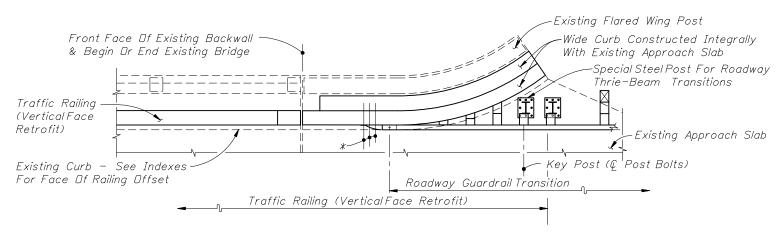
# PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)





SEE INDEX NO. 482 - SCHEME 1



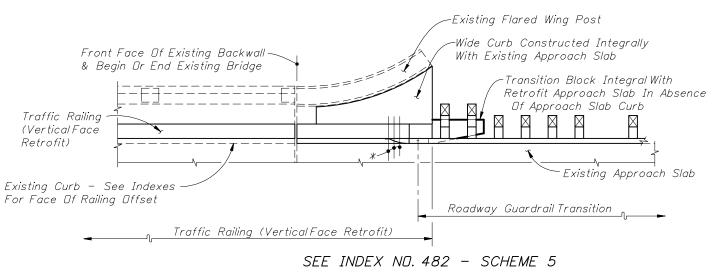


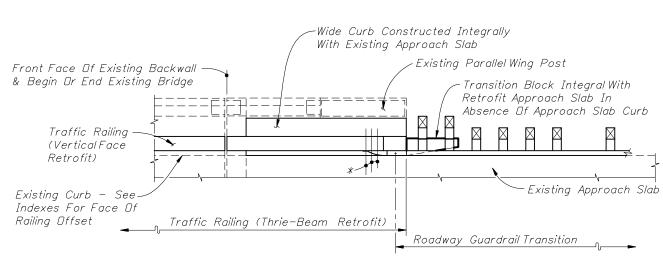
SEE INDEX NO. 482 - SCHEME 4

\*21" x 12" x  $\frac{5}{8}$ " Thrie-Beam Terminal Connector Plate (Back-Up Plate), And  $\frac{7}{8}$ " 0 x 12" Long HS Hex Bolts And Nuts (5 Regd.) With 21/4" OD Plain Round Washers Under Heads And Nuts

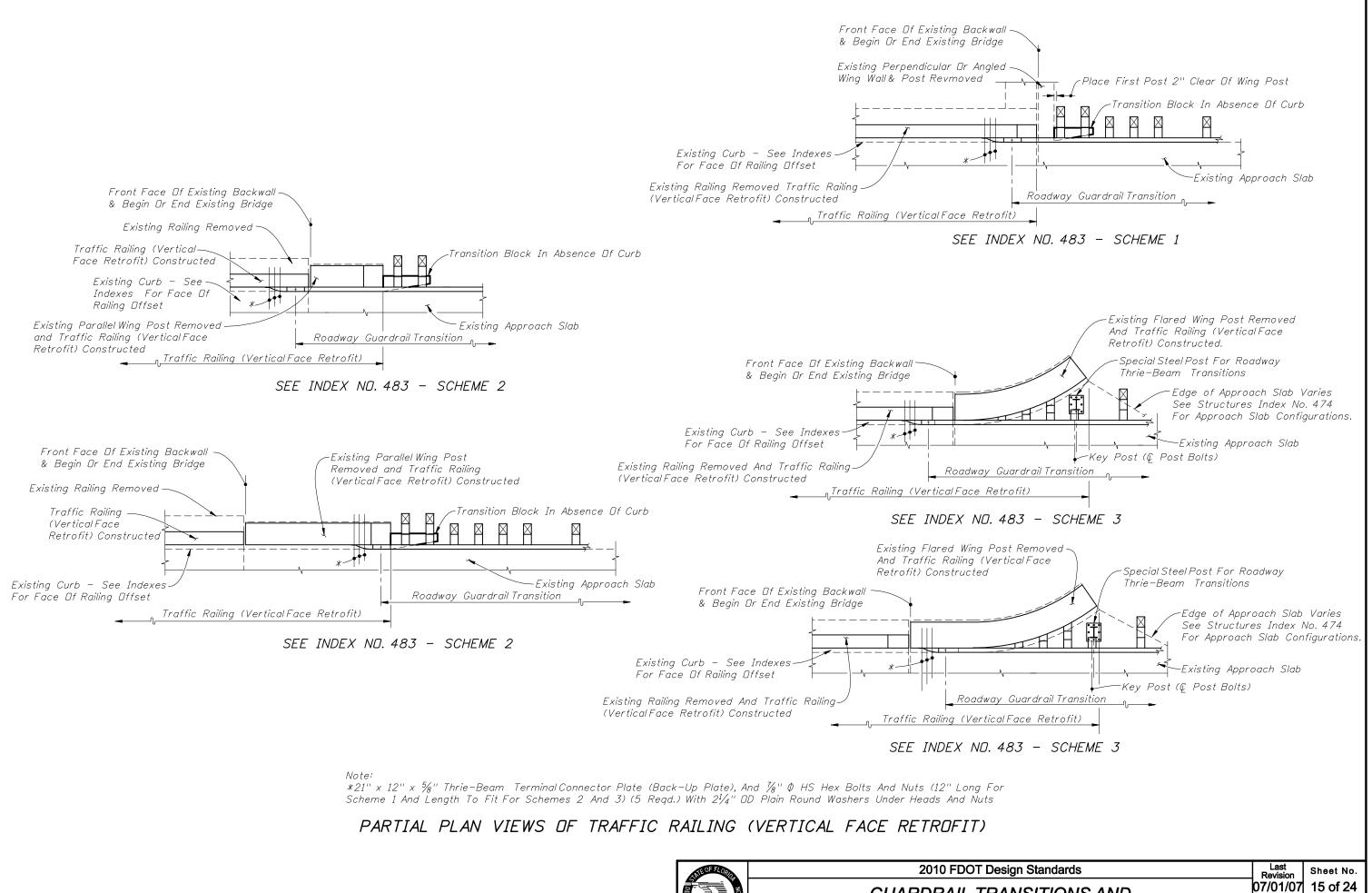
# PARTIAL PLAN VIEWS OF TRAFFIC RAILING (VERTICAL FACE RETROFIT)





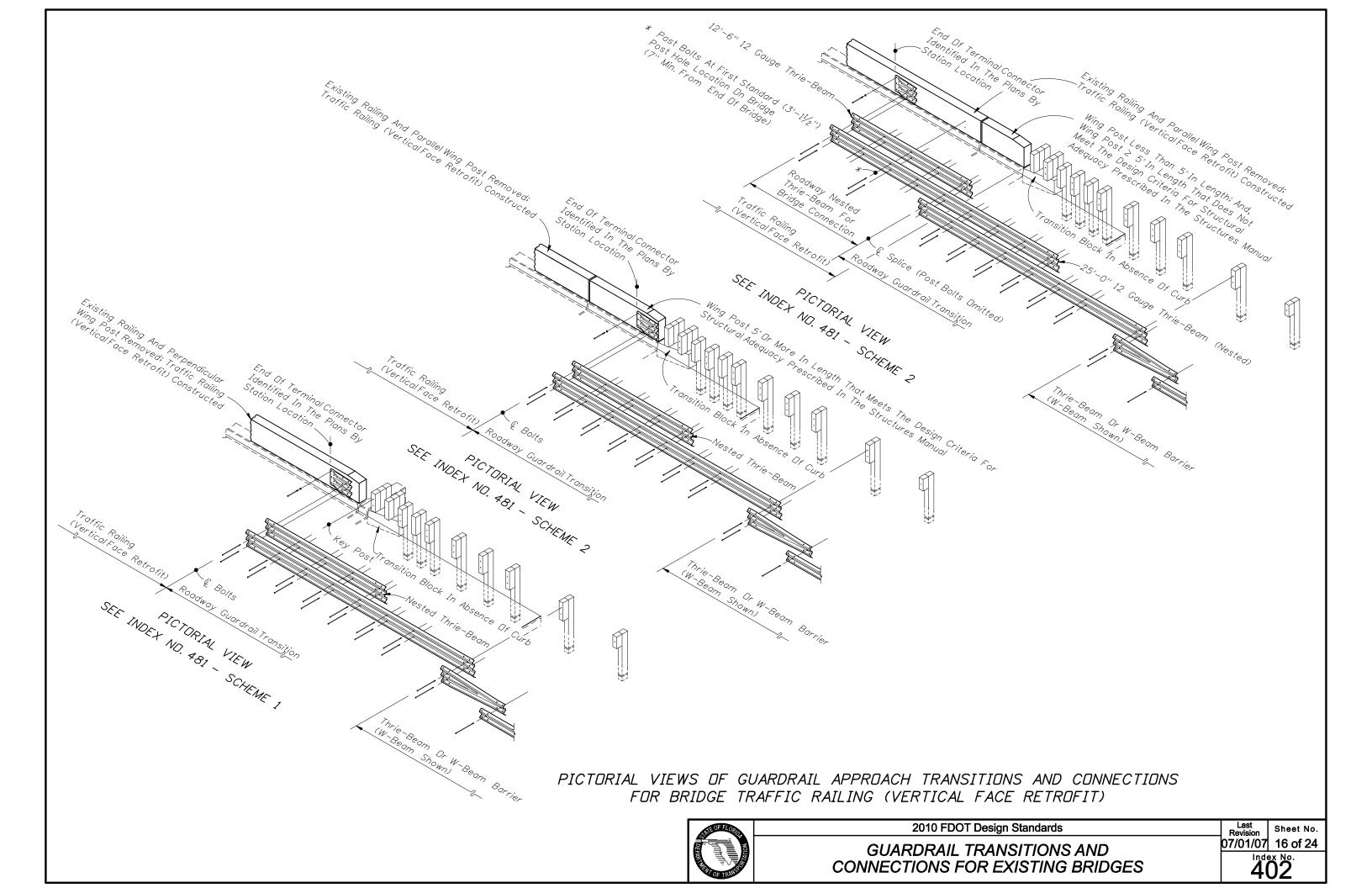


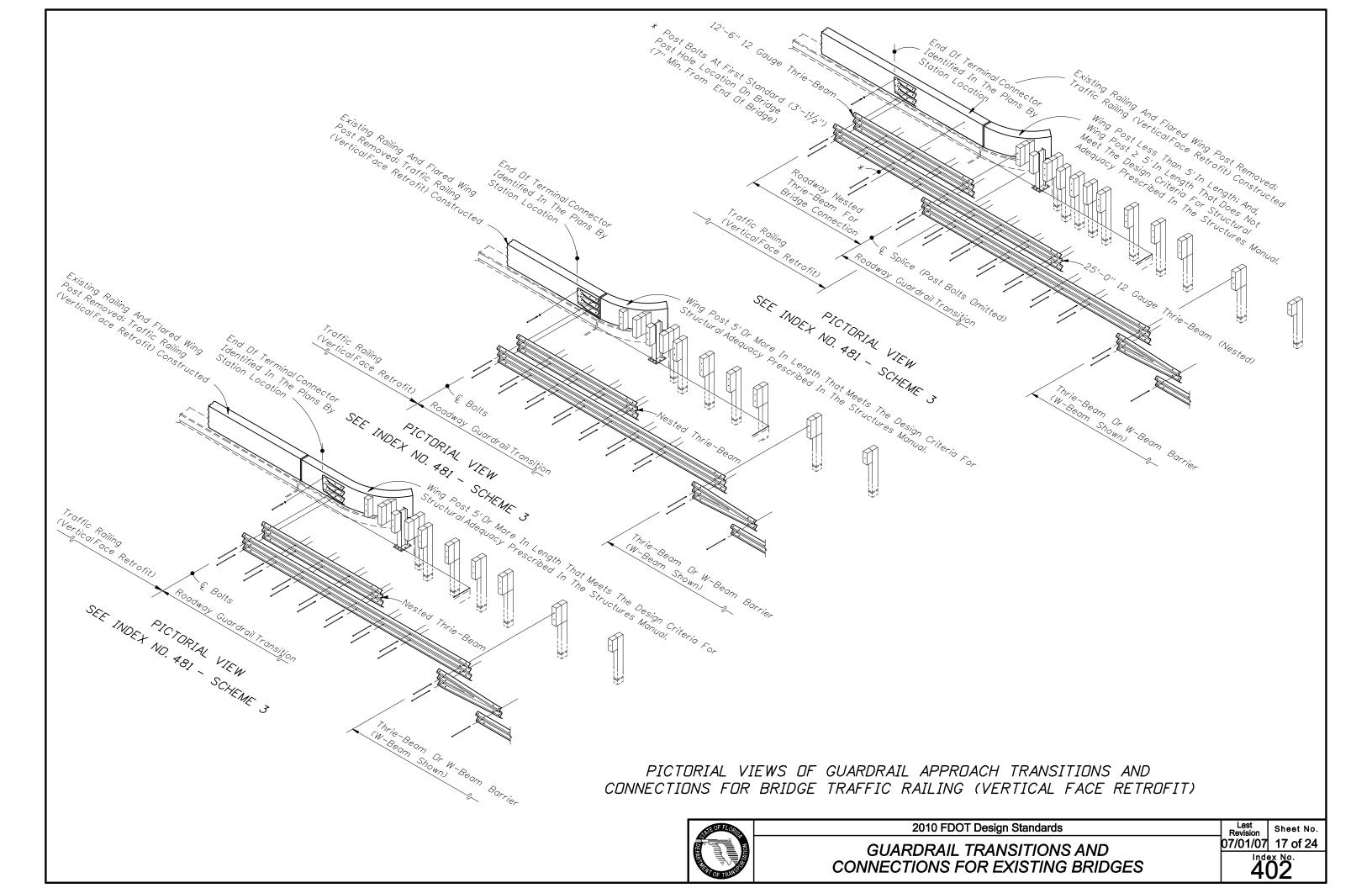
SEE INDEX NO. 482 - SCHEME 5

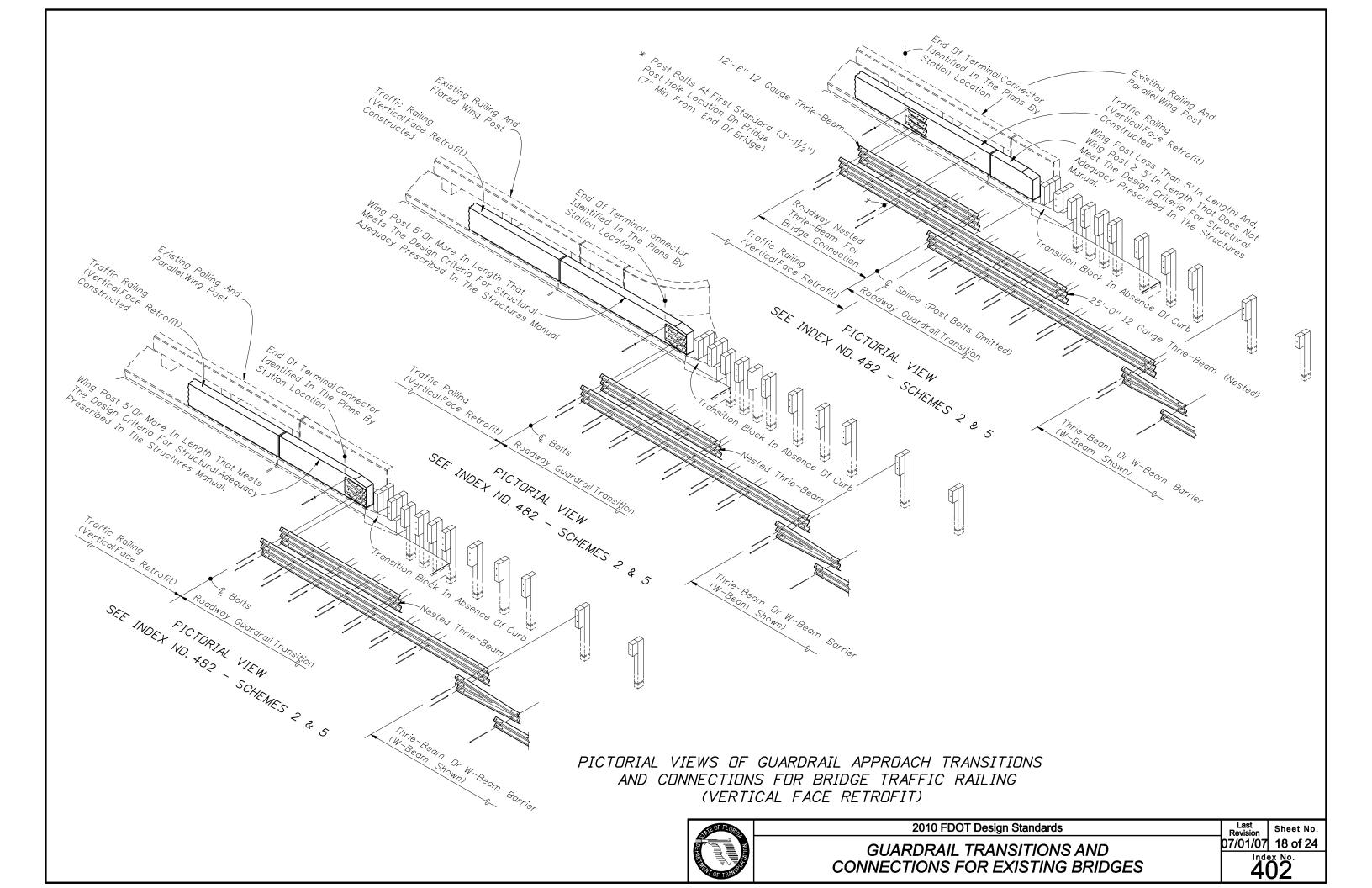


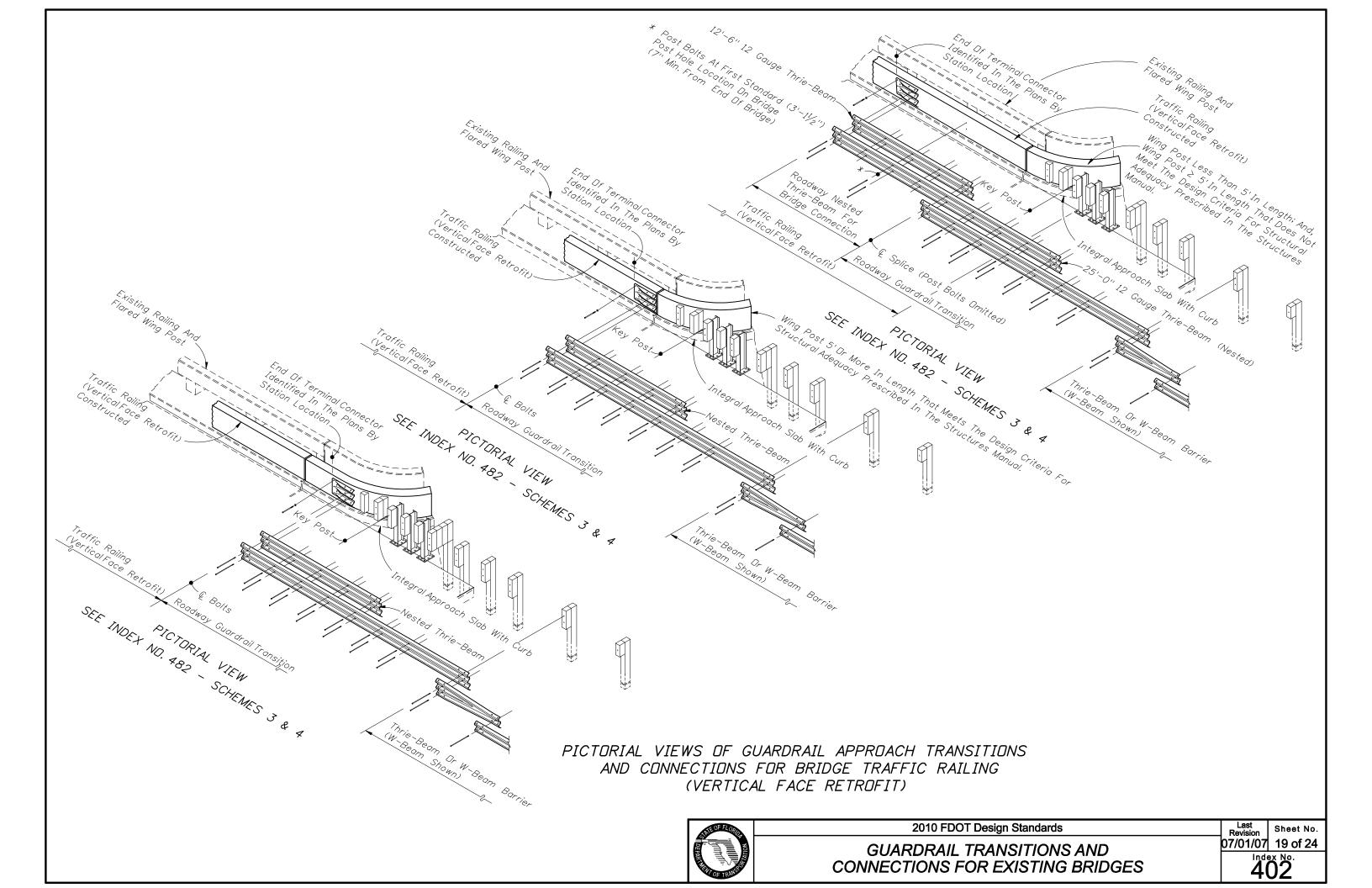
2010 FDOT Design Standards

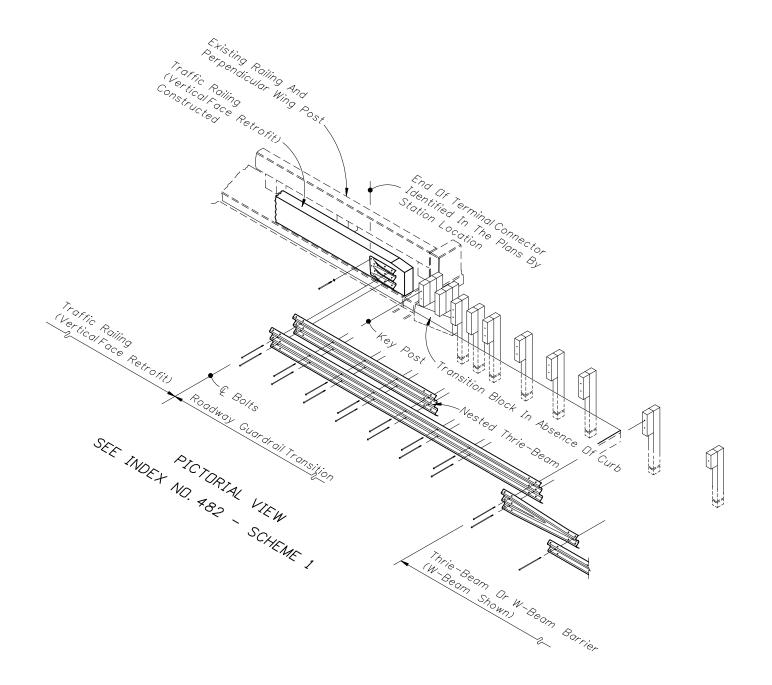
GUARDRAIL TRANSITIONS AND
CONNECTIONS FOR EXISTING BRIDGES





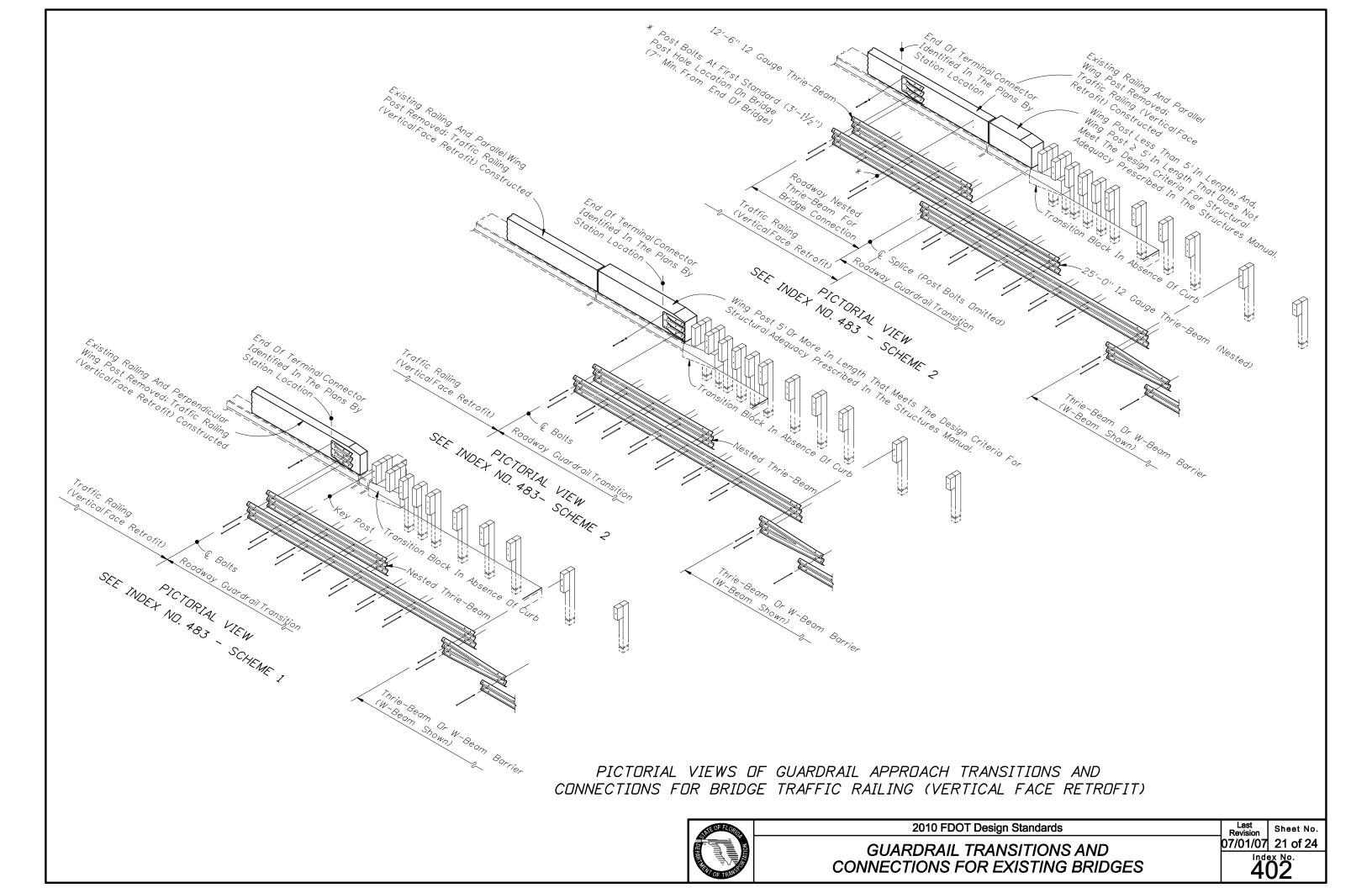


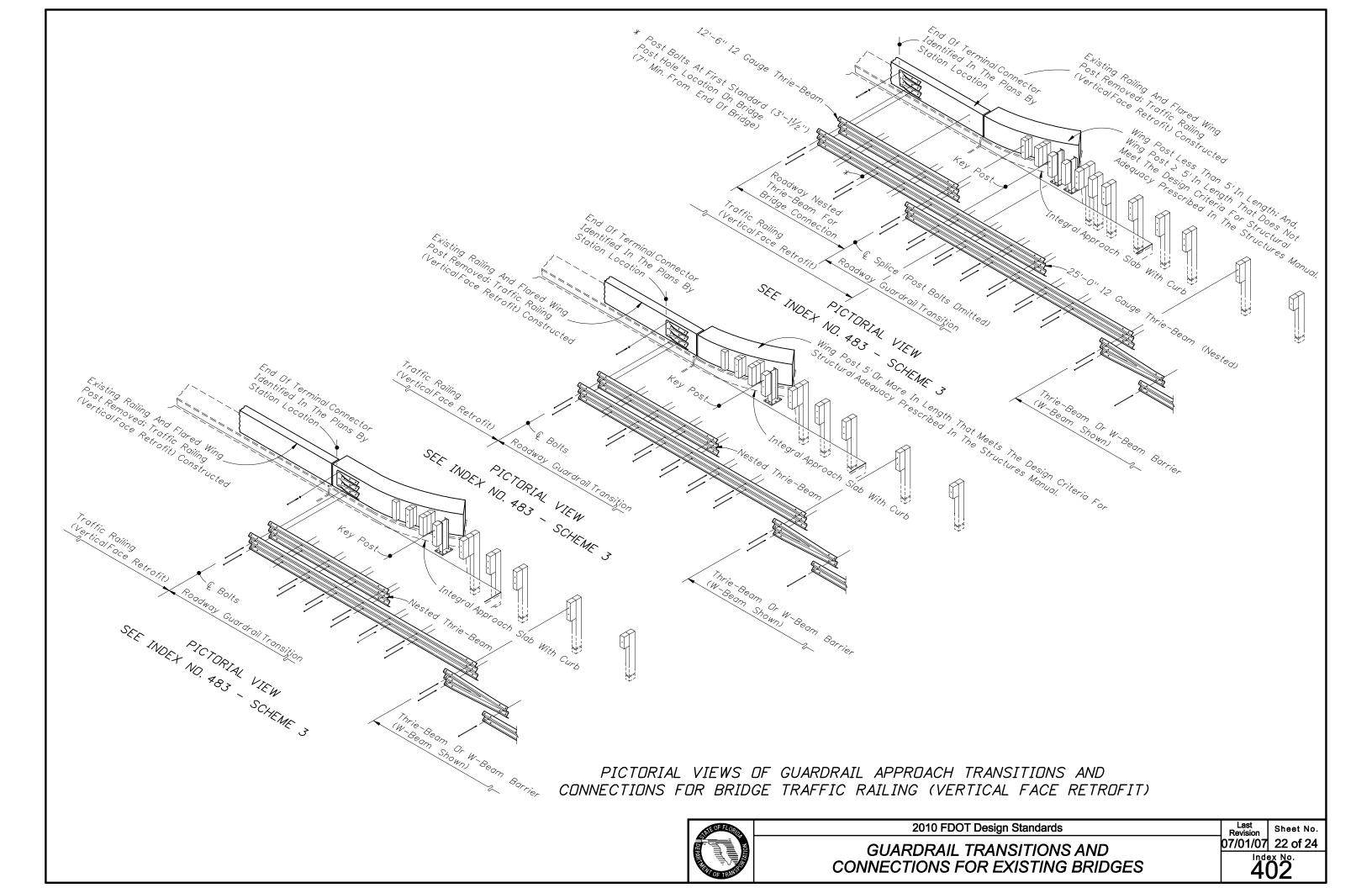


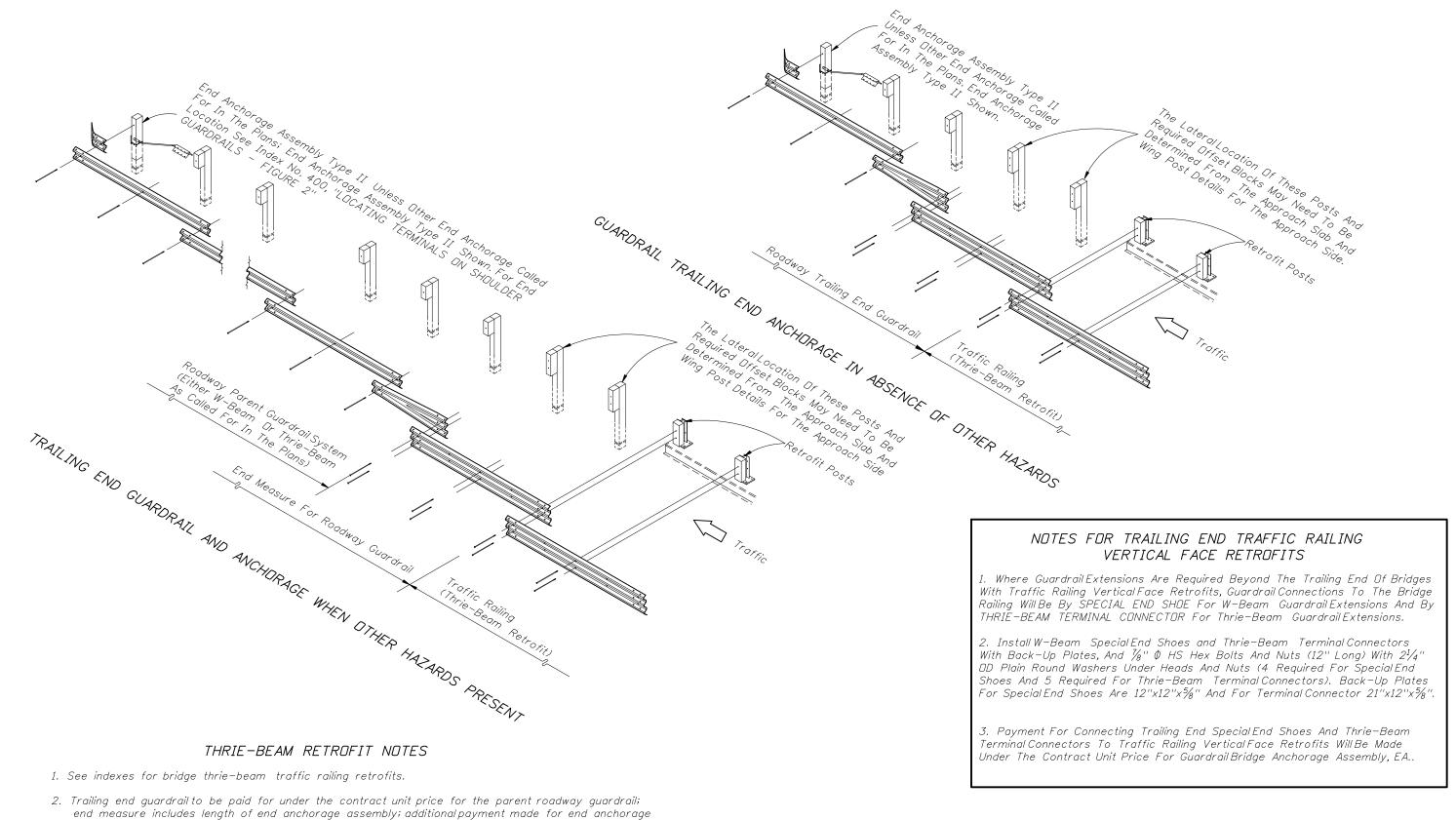


PICTORIAL VIEWS OF GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR BRIDGE TRAFFIC RAILING (VERTICAL FACE RETROFIT)





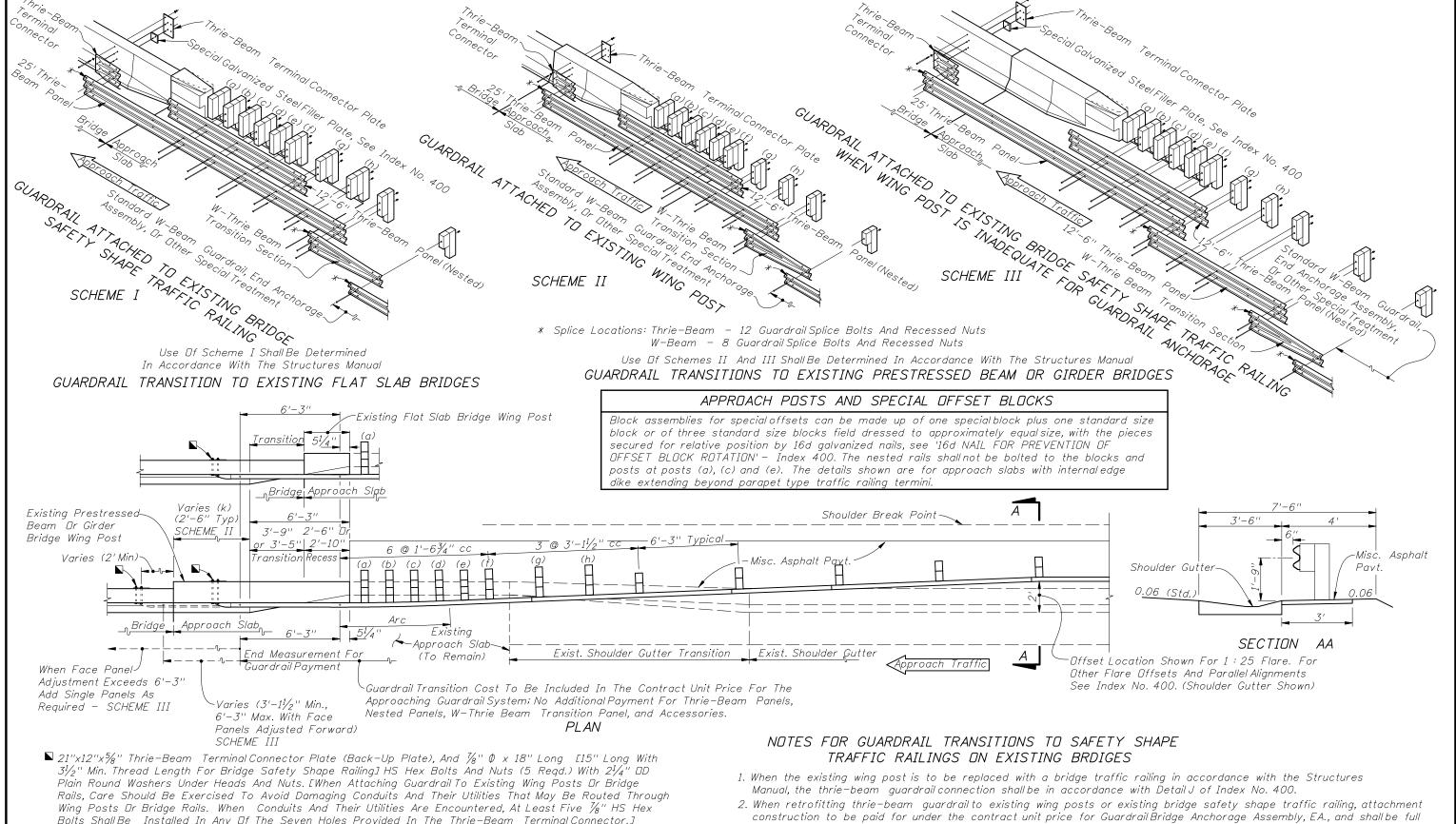




assembly. No additional payment for connecting roadway thrie-beam to bridge thrie-beam retrofit.

TRAILING END GUARDRAIL AND ANCHORAGE FOR BRIDGE TRAFFIC RAILING (THRIE BEAM RETROFITS)





Bolts Shall Be Installed In Any Of The Seven Holes Provided In The Thrie-Beam Terminal Connector.]

compensation for bolt hole construction, terminal connector, terminal connector plate(s) and bolts, nuts and washers.

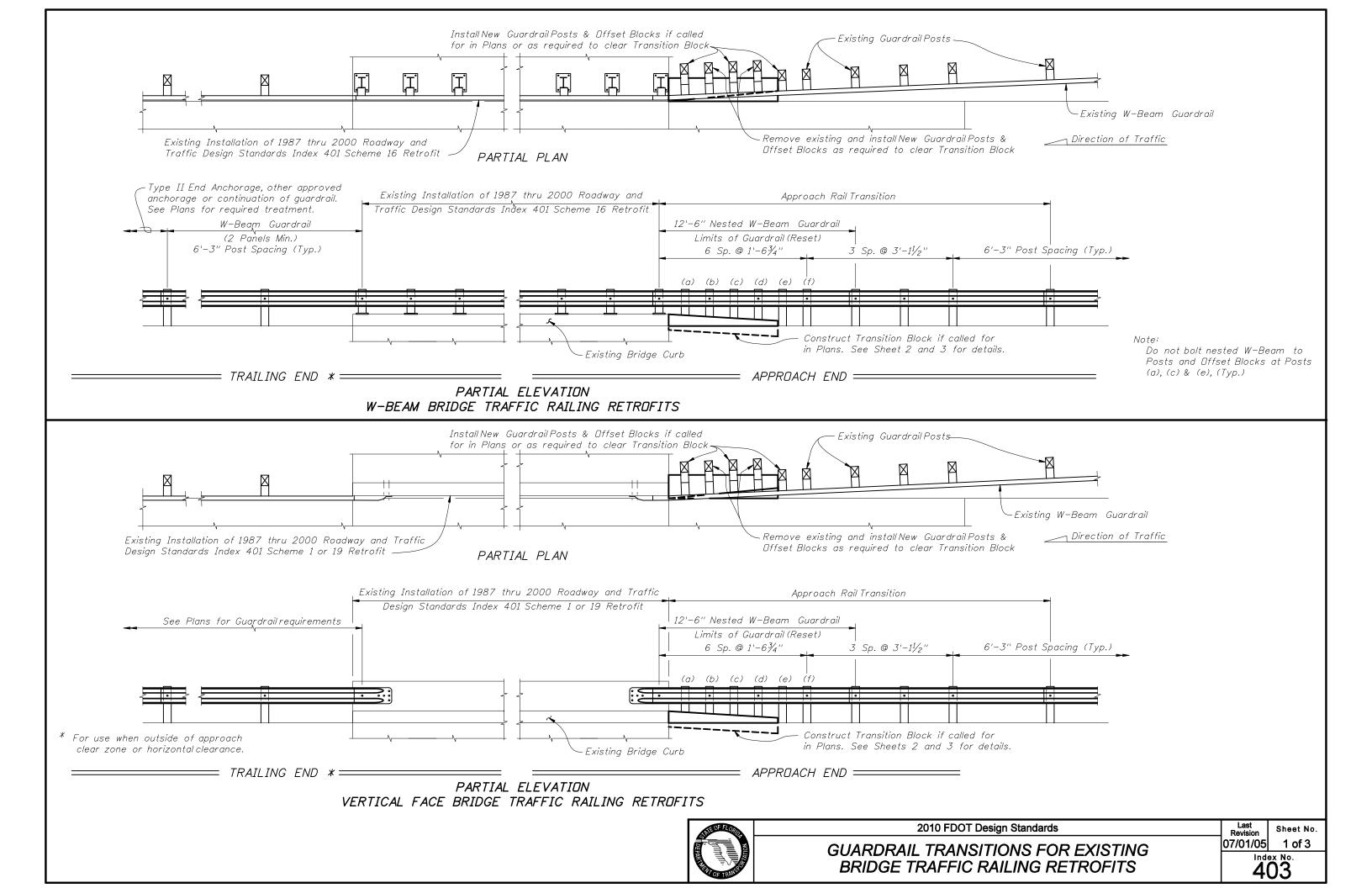
GUARDRAIL APPROACH TRANSITIONS AND CONNECTIONS FOR EXISTING FLAT SLAB, PRESTRESSED BEAM AND GIRDER BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH

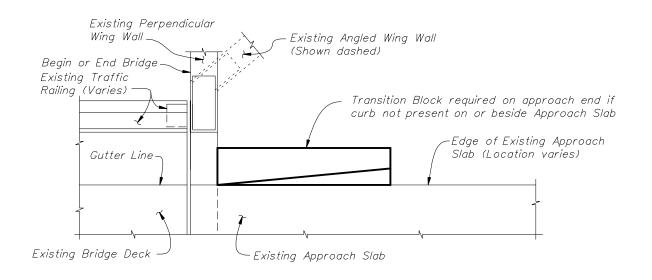


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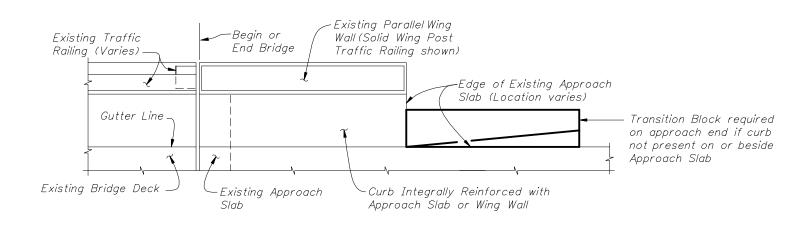
**GUARDRAIL TRANSITIONS AND** CONNECTIONS FOR EXISTING BRIDGES

Sheet No. 07/01/07 24 of 24

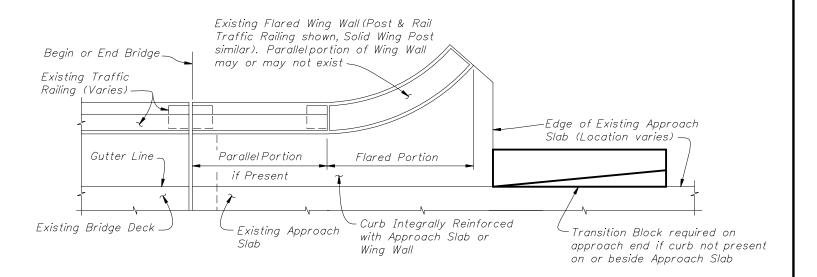








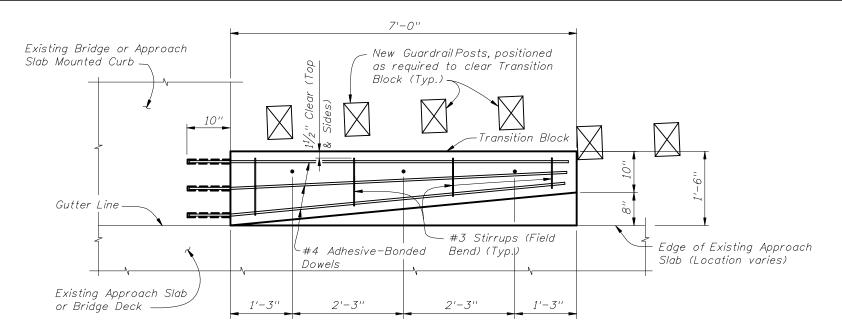
PARTIAL PLAN VIEW OF EXISTING BRIDGE WITH PARALLEL WING WALLS
AND INTEGRALLY REINFORCED APPROACH SLAB CURBS
(APPROACH SLAB WITH DETACHED CURBS OR SIDEWALK SIMILAR)



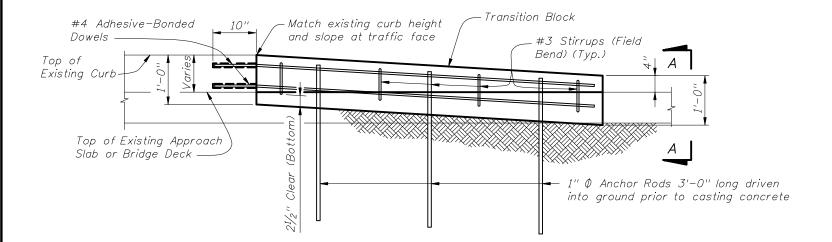
PARTIAL PLAN VIEW OF EXISTING BRIDGE WITH FLARED WING WALLS AND PARALLEL INTEGRALLY REINFORCED APPROACH SLAB CURBS (APPROACH SLAB WITH DETACHED CURBS OR SIDEWALK SIMILAR)

CROSS REFERENCE:
For Trainsition Block Details,
Quantities and reinforcement
see Sheet 3.



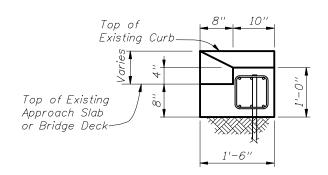


PLAN VIEW OF TRANSITION BLOCK (GUARDRAIL NOT SHOWN FOR CLARITY)

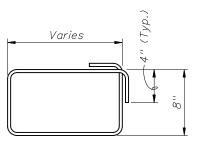


ELEVATION OF TRANSITION BLOCK (GUARDRAIL AND POSTS NOT SHOWN FOR CLARITY)

ESTIMATED QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete Class II (Miscellaneous)	CY	0.4	
Reinforcing Steel (Roadway)	LB	61	
Guardrail (Reset)	LF	12.5	



END VIEW A-A



#3 STIRRUP (FIELD BEND)

NOTES:

 $\textit{CDNCRETE: Concrete for Transition Blocks shall be \textit{Class II (Miscellaneous)}.}$ 

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

ANCHOR RODS: Steel Anchor Rods shall be ASTM A36, ASTM A709 Grade 36 or ASTM A615 Grade 60 hot-dip galvanized in accordance with Specification Section 962.

W BEAM GUARDRAIL: Guardrail components and installation shall be in accordance with Design Standards Index 400.

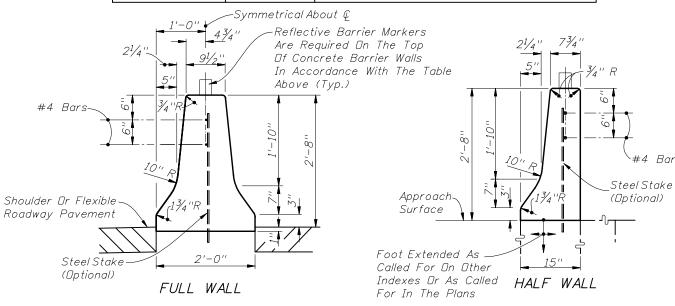
ADHESIVE-BONDED DOWELS: Adhesive Bonding Material Systems for Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416.

Adhesive Bonded Dowels are shown installed in an existing curb or sidewalk integrally reinforced with Approach Slab, Wingwall or Bridge Deck. For installations in existing detached curbs or sidewalks, install dowels in available sound concrete.

PAYMENT: Payment for Guardrail work will be made under Pay Item Guardrail (Reset) (LF). Payment for Transition Block will be made under Pay Items Concrete Class II (Miscellaneous) (CY) and Reinforcing Steel (Roadway) (LB).



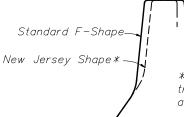
REFLECTIVE BARRIER MARKER SPACING ON WALL				
Distance- Edge of Travel		REMARKS		
Lane to Barrier Wall (ft)	Spacing (Ft.)	1. Reflectors shall conform to Section 993 of the Standard Specifications.		
< 4 '	40'	2. Reflector color (white or yellow) shall		
4' to 8'	80'	conform to the color of the near edgeline. 3. The cost for reflectors shall be included in		
> than 8'	none required	the contract unit price for barrier wall.		



For concrete barrier wall details at piers, highway lighting and guardrail connections, see other sheets of this Index.

Standard barrier to be paid for under the contract unit price for Median Concrete Barrier Wall, LF.

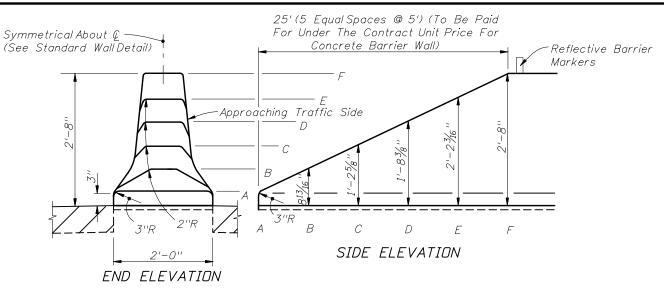
### STANDARD BARRIER WALL SECTIONS



\* Where standard F-Shape walls abut existing NJ Shape walls, face transitions of not less than 5' in length shall be constructed at the adjoining end of the F-Shape wall.

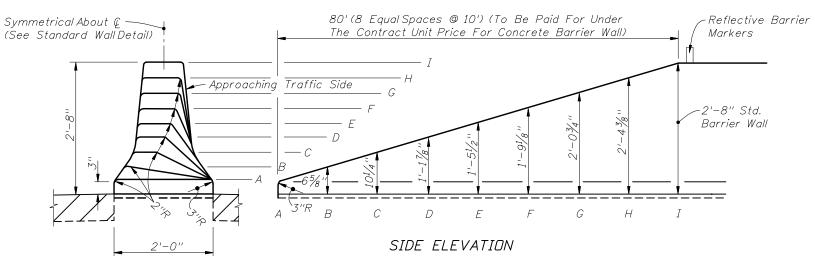
#### WALL FACE SAFETY SHAPES

- 1. Class II concrete shall be used for all reinforced and plain (nonreinforced) concrete barrier walls; except, in moderately and extremely aggressive environments, Class IV concrete shall be used. All reinforcing steel with undesignated size shall be #4 bars. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specifications, unless other finish called for in plans. The surfaces shall have a Class 5 Applied Finished Coating in accordance with Section 400 only when called for in the plans.
- 2. Concrete barrier wall terminal notes for design speeds ≥50 mph.
  - a. Terminated outside clear zone of the approach traffic with 'Detail II' end treatment.
  - b. Terminated within a shielded location.
  - c. Terminal protection by the use of a crash cushion system.
  - d. Terminated in conjunction with a suitably designed transition to another barrier.
- 3. Expansion joints in wall required only at bridge ends and/or at locations where wall is an integral part of existing or proposed concrete slab; wall joints are to match an existing or proposed expansion joint.
- 4. When the barrier is installed adjacent to the pavement the top 12" of the subgrade shall be compacted to at least 100% of the density as defined in the AASHTO T-99 specifications.



TO BE USED ONLY WHERE TERMINAL LOCATED CLEAR ZONE WIDTH FROM EDGE OF THE NEAR APPROACH TRAFFIC LANE.

## CONCRETE BARRIER WALL TERMINAL DETAIL II



DESIGN SPEED 45 MPH OR LESS END ELEVATION CONCRETE BARRIER WALL TERMINAL FOR NARROW MEDIAN

#### GENERAL NOTES

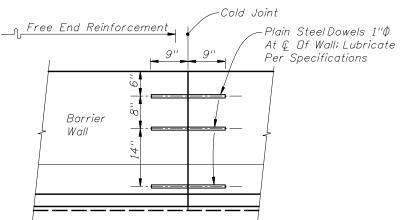
#### DETAIL III

- 5. Cast-in place barrier wall normally will be a continuous pour without transverse contraction joints. Cast-in-place segments with a length <40' shall be joined to adjacent sections by doweling. See DetailB on Sheet 2.
- 6. Precast construction is allowed as an alternate to cast-in-place construction.
  - a. Wall segments <40' in length shall be joined by a transverse joint in accordance with Details C & D on Sheet 2. The minimum segment length is 20'.
  - b. Bedding of the precast sections shall be facilitated by the use of sand-cement grout or equalmethod to assure uniform bearing.
  - c. Reinforcement may be required for handling stresses.
- 7. On roadways designated for reverse laning all downstream, ends that are not shielded or outside the clear zone shall be marked by Type 3 Object Markers.
- 8. Cost of reinforcing steel and reflective barrier markers shall be included in the contract unit price for concrete barrier wall. See individual details for pay item information.
- 9. For barrier wall inlet details see Indexes Nos. 217, 218 and 219.
- 10. Concrete barrier wall with New Jersey Safety Shape may not be substituted for the Standard F Shape Barrier.



Sheet No.

1ndex No.

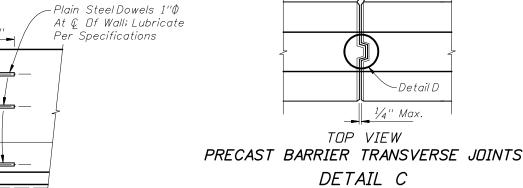


DOWELED TRANSVERSE CONSTRUCTION JOINT WHEN ABUTTING SEGMENT(S) LESS THAN 40' IN LENGTH DETAIL B

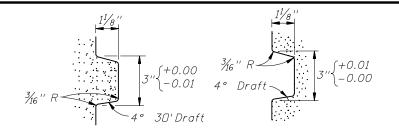
Shoulder Pavement-

Concrete

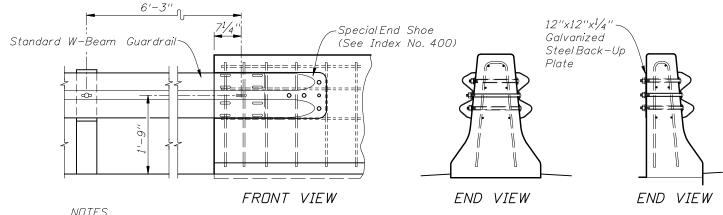
Barrier Wall



Edge Of Traveled Way



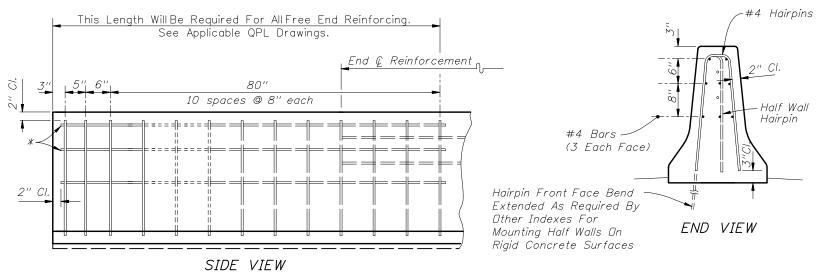
TOP VIEW STRAIGHT TONGUE AND GROOVE DETAIL D



 $^{-1}/_{4}$ " Chamfer Typ.

- 1. End of wall flush mounted connections are not applicable to two-lane two-way facilities. See Sheets 20, 24, and 25 for trailing end connections on two-lane two-way facilities and for approach quardrail connections.
- 2. Trailing guardrail connections to double face safety shaped walls will be under one of the following traffic conditions and mounting methods:
  - (a) One-way traffic trailing condition one side only flush mount with flat steel back-up plate on back side.
  - (b) One-way traffic trailing condition both sides flush mount both sides.
  - (c) For trailing condition one side and approach traffic condition opposite side see "Median Barrier Wall" mounting, Sheet 25.

## W-BEAM GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL TRAILING ENDS



\* Note: Free end reinforcement required for nonreinforced walls at the following locations: All exposed ends; abutting ends of true joints; ends with quardrail connections; ends with redirective crash cushion connections; and, ends connecting to bridge traffic rails or other rigid barrier walls.

## FREE END REINFORCEMENT



2010 FDOT Design Standards

Sheet No.

CONCRETE BARRIER WALL

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Edge Of Traveled Way 2' Misc. Asphalt Pavement (2" Thick ∕@ Const. ∠25' (See Detail No. II.

CONCRETE BARRIER WALL TRANSITION BETWEEN WIDE AND

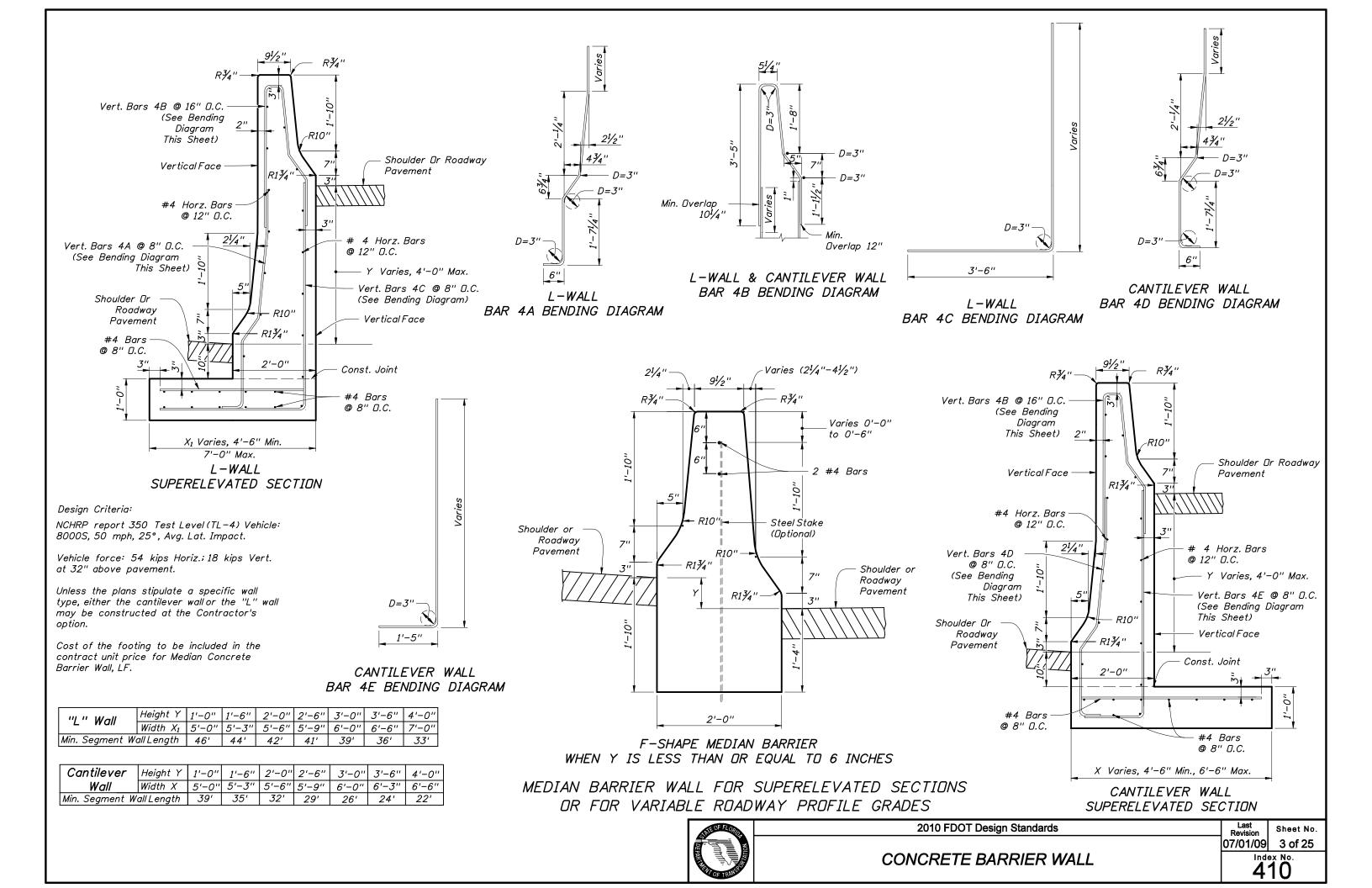
NARROW MEDIANS WHEN BARRIER WALL END LOCATED DUTSIDE APPROACH CLEAR ZONE OR HORIZONTAL CLEARANCE

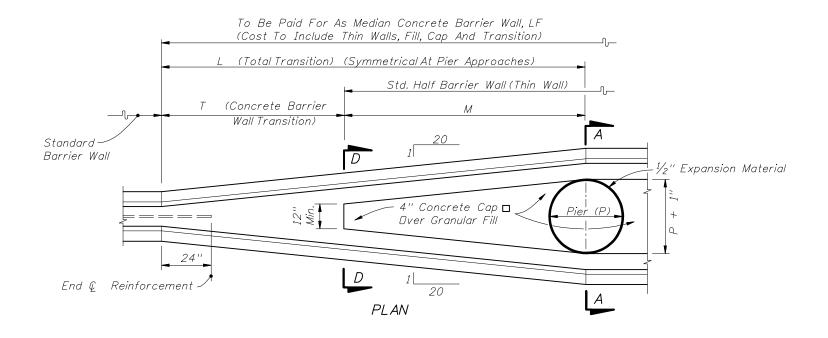
✓ Direction Of Traffic Edge Of Traveled Way Extended Shoulder Not Steeper Than 1:10 15° Or Flatter

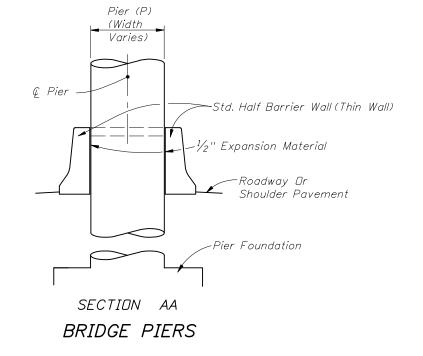
Crash Cushion --Misc. Asphalt (Type Varies, See Plans Pavt. Extended Shoulder Concrete Barrier Wall Break Point (Full, Half Or Trapezoidal Wall)

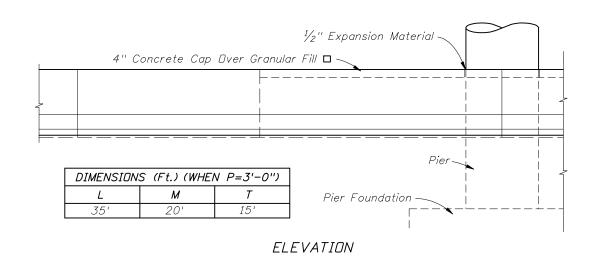
SHOULDER TREATMENT WHEN CRASH CUSHIONS SHIELDING CONCRETE BARRIER WALL END LOCATED INSIDE APPROACH CLEAR ZONE OR HORIZONTAL CLEARANCE

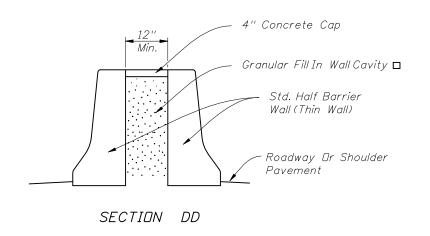
DETAIL A







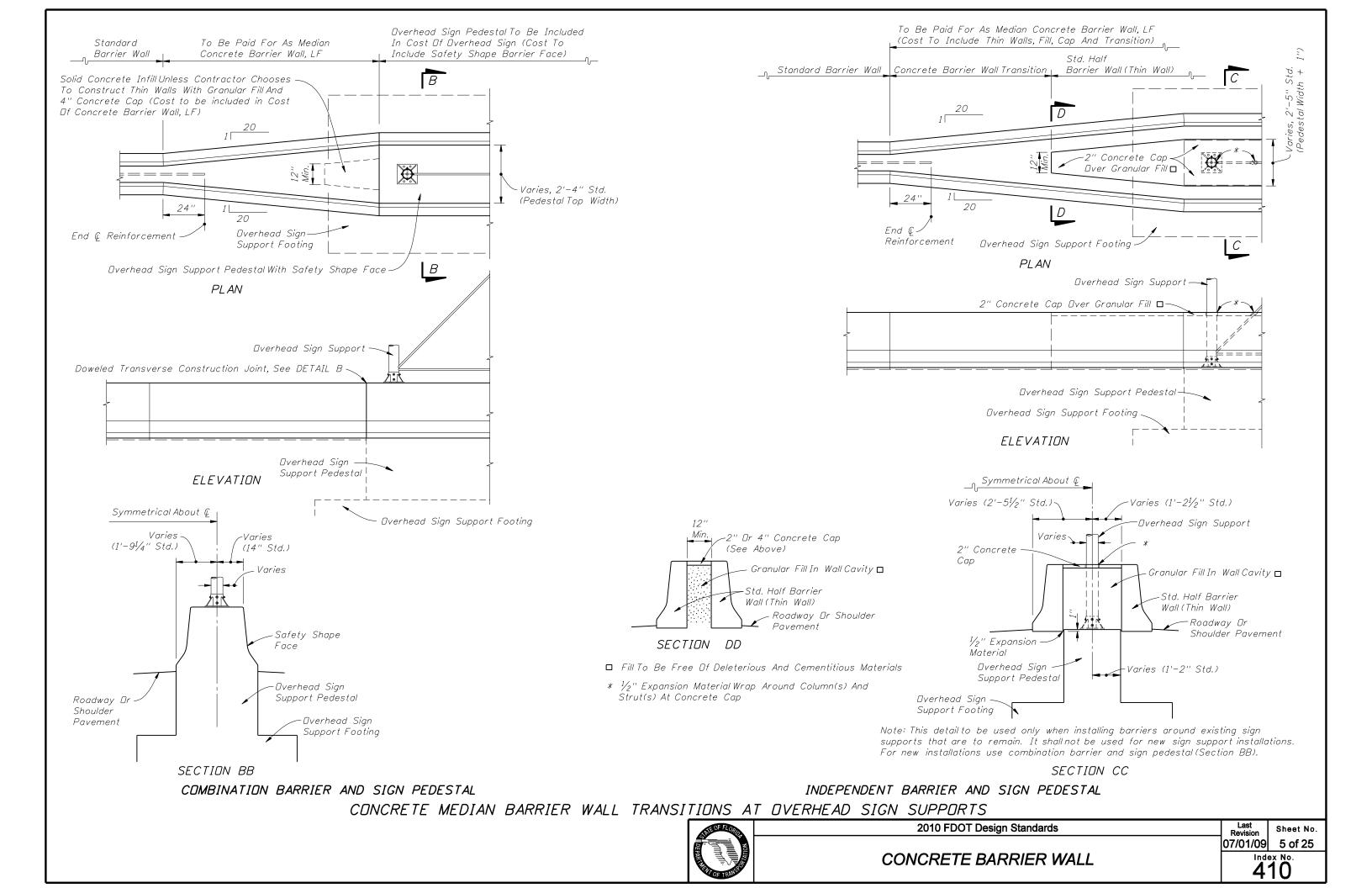


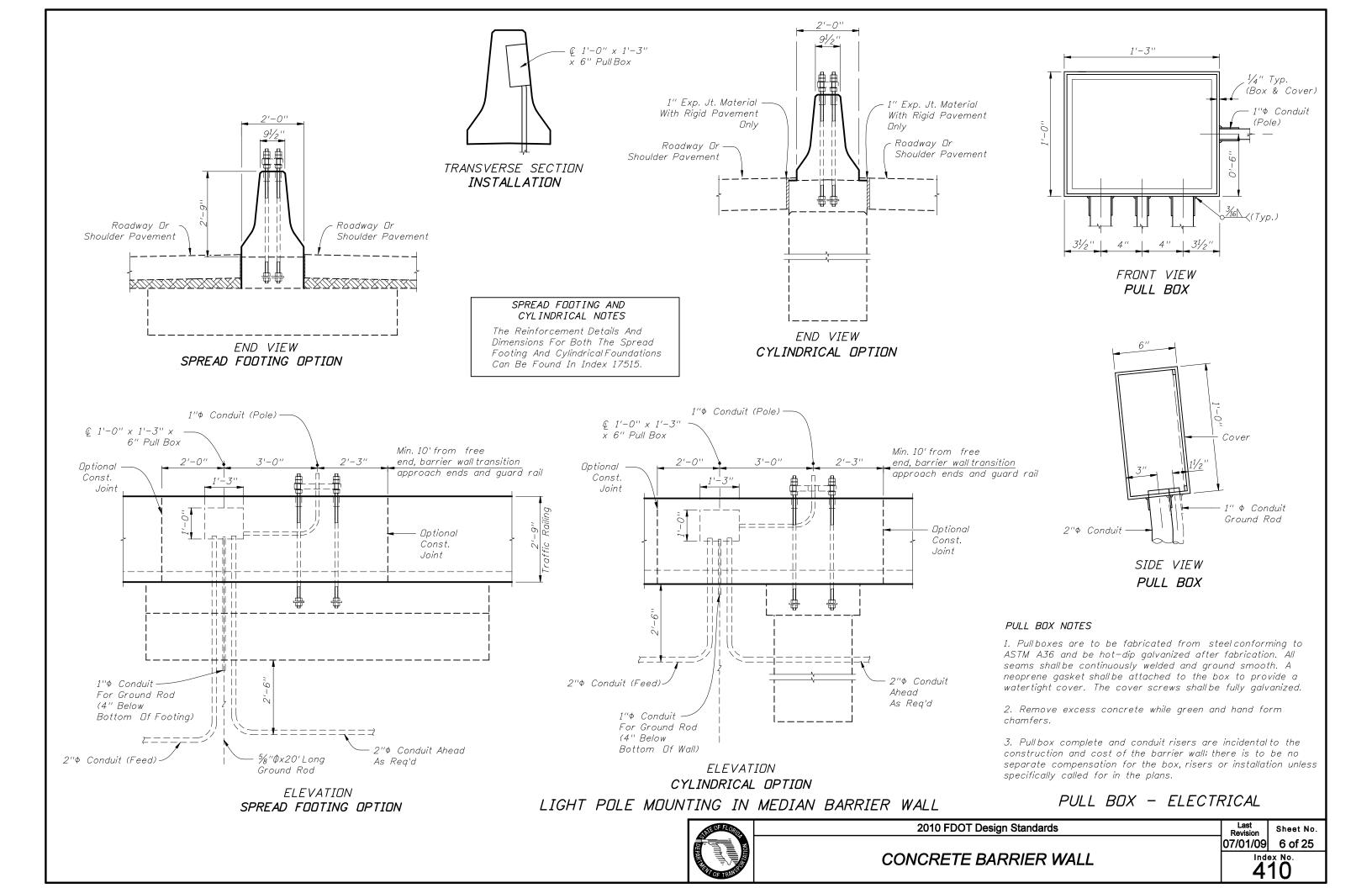


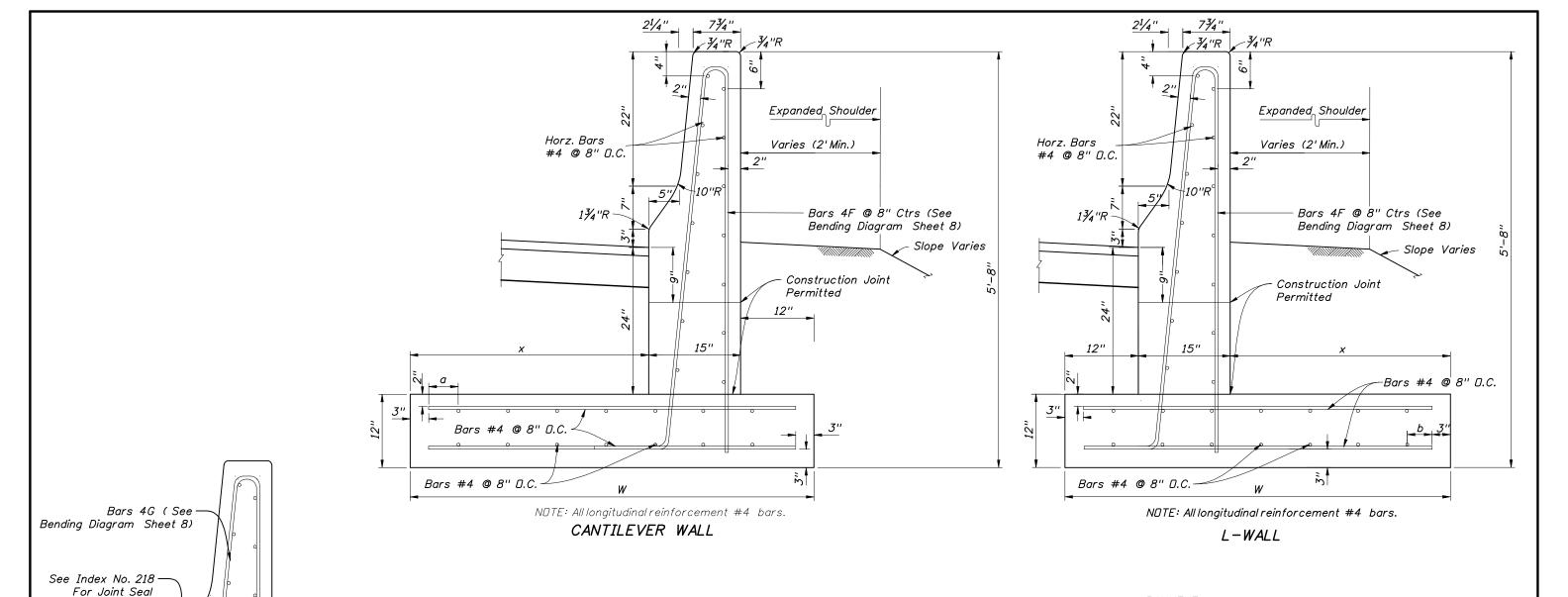
□ Fill To Be Free Of Deleterious And Cementitious Material

## CONCRETE MEDIAN BARRIER WALL TRANSITIONS AT BRIDGE PIERS

STATE OF FLORIDA	2010 FDOT Design Standards	Last Revision	Sheet No.
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	CONCRETE BARRIER WALL	Inde	ex No.
OF TRANS		4	10







	DIMENSIONS AND QUANTITIES										
	CANT	ILEVE	-R	WALL				L - 1	VAL	L	
Length* Of Barrier Wall	W	×	а	Class II Conc. CY Per Lin. Ft.	Rein. Steel Lbs. Per Lin. Ft.	Length* Of Barrier Wall	W	х	Ь	Class II Conc. CY Per Lin. Ft.	Rein. Steel Lbs. Per Lin. Ft.
≥ 65′	3'-3"	1'-0"	5"	0.30	31	≥ 60′	3'-6"	1'-3"	7"	0.31	32
57' to 64'	3'-9"	1'-6"	3"	0.32	34	50' to 59'	4'-0"	1'-9"	5"	0.33	34
50' to 56'	4'-3"	2'-0"	3"	0.33	36	40' to 49'	4'-9"	2'-6"	6"	0.35	<i>37</i>
41' to 49'	5'-0"	2'-9"	7"	0.36	38	35' to 39'	5'-3"	3'-0"	4"	0.37	39
36' to 40'	5'-6"	3'-3"	5"	0.38	40	30' to 34'	5'-9''	3'-6"	2"	0.39	42
28' to 35'	6'-6"	4'-3"	3"	0.42	45	25' to 29'	6'-6"	4'-3"	3"	0.42	45
25' to 27'	7'-0"	4'-9"	7"	0.44	46	20' to 24'	7'-6"	5'-3"	2"	0.45	49

Barrier Wall Inlet

#4 Bars, 10' In Length

Centered On Inlet

REINFORCING STEEL MODIFICATIONS

AT BARRIER WALL INLETS (INDEX NO. 218)

Quantities shown are for information only. For method of payment see payment note. Barrier wall inlets (Index 218) shall be isolated from the barrier wall stem and footing by 1" expansion material. \* All walls may be made up of segments of 20' or more in length provided the segments are joined by a transverse joint in accordance with Detail B, Sheet 2. Segments shall have dimensions same as walls above.

REINFORCED CONCRETE BARRIER WALL (SHOULDER)

### PAYMENT:

Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder), LF.

### DESIGN NOTES:

Reduce the vertical steel spacing to 4 inches D.C. a distance of 4 feet each side of all cold joints.

Use of this barrier wall should be limited to special applications such as hazard encroachment into the clear zone where barrier wall deflection, rotation or translation cannot be tolerated; example hazards to consider are as follows:

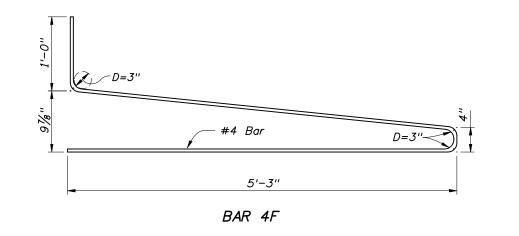
(a) Structure supporting piers, bents and pylons (b) Pumping, metering, control or other similar critical stations (c) Quarries (d) Intolerable vertical drops (e) Historic structures or monuments (f) Rail transit travel way or passenger station (g) Other similar occupancies

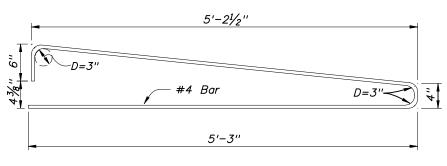
NCHRP report 350 Test Level (TL-4) Vehicle: 8000S, 50 mph, 25°. Vehicle force applications: 18 kips Vert. at top of railing; 54 kips Horiz. at 32" above pavement.

Sheet No.

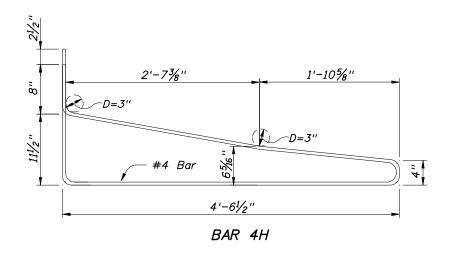
07/01/09 7 of 25 Index No. 410

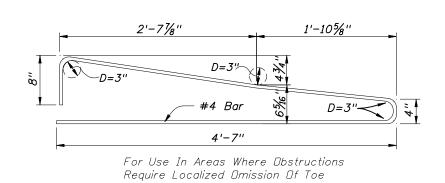






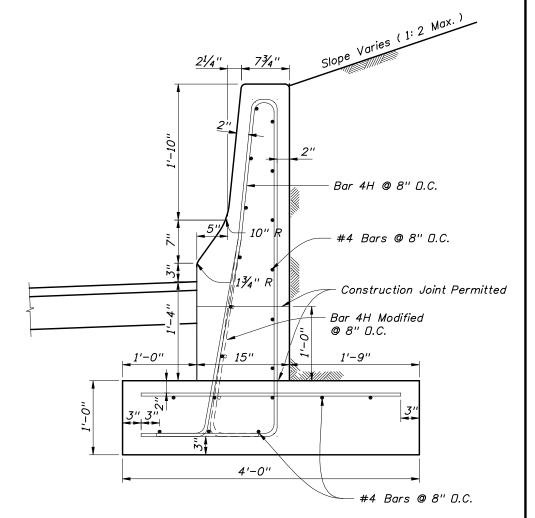
BAR 4G





BAR 4H MODIFIED

## BENDING DIAGRAMS



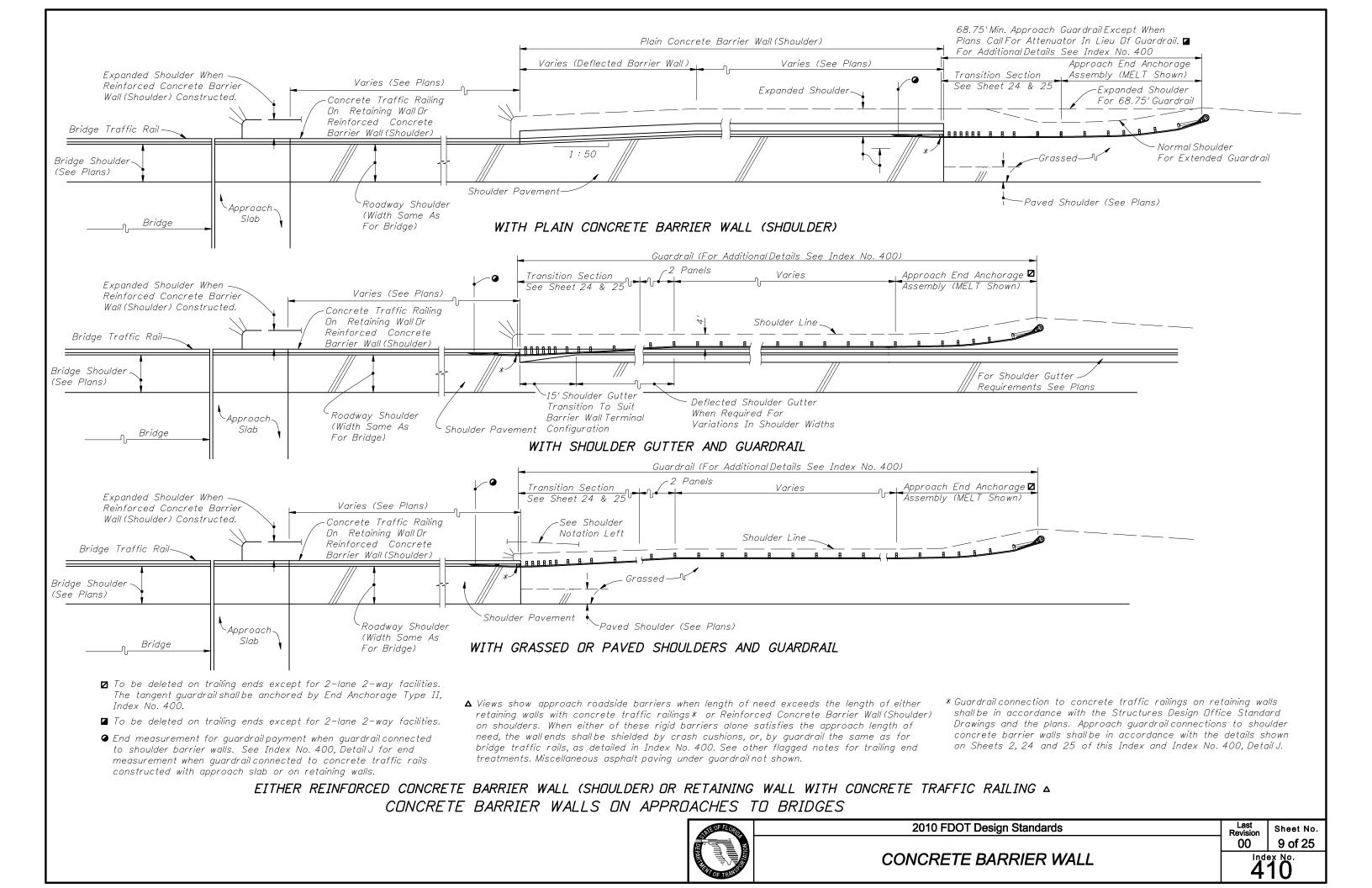
Note: All longitudinal reinforcement #4 bars. Minimum segment length for this wall is 20 feet. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid–Retaining), LF.

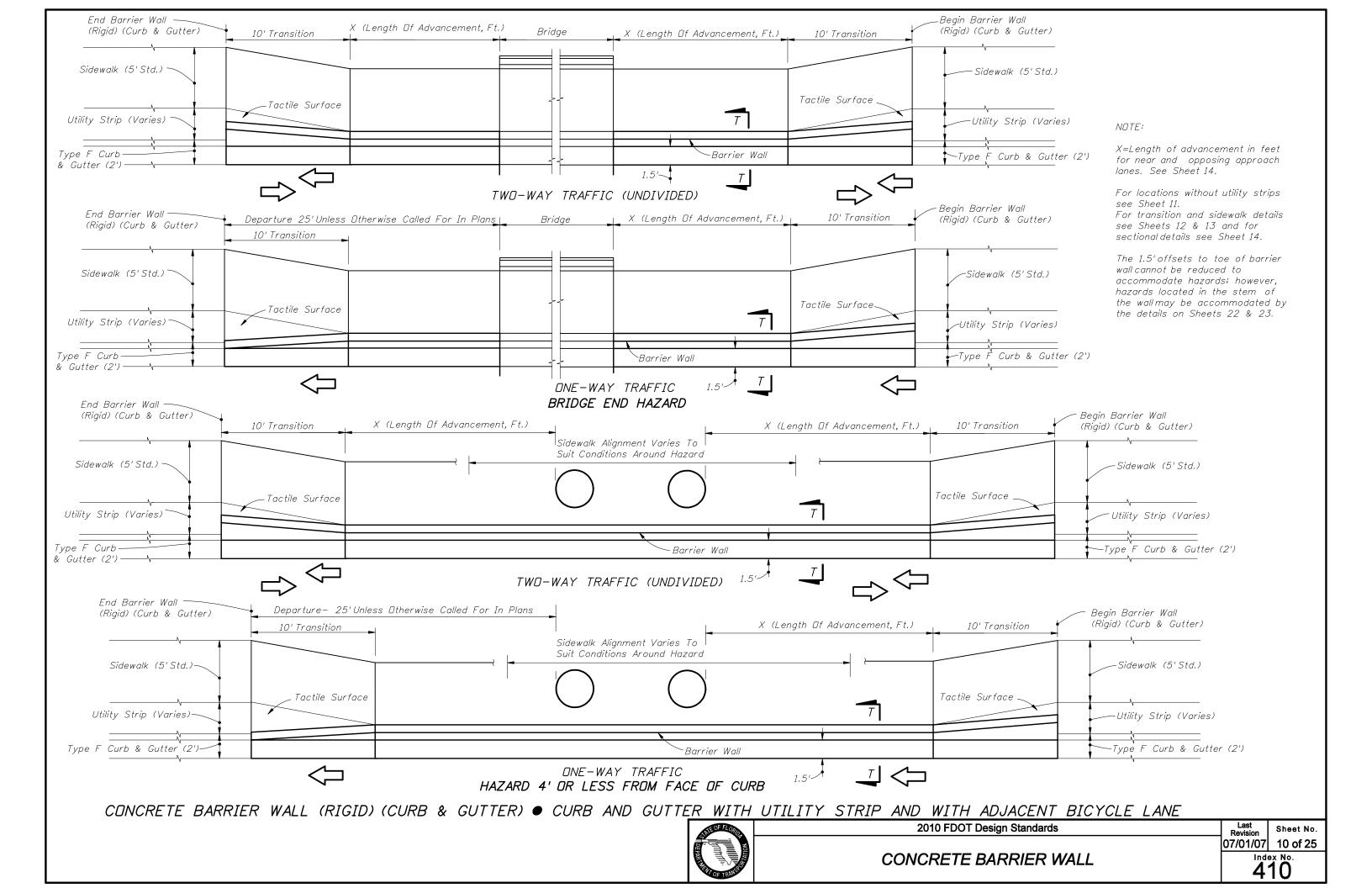
QUANTITIES: Class II Concrete 0.29 CY/LF Reinforcing Steel 28.6 LBS/LF

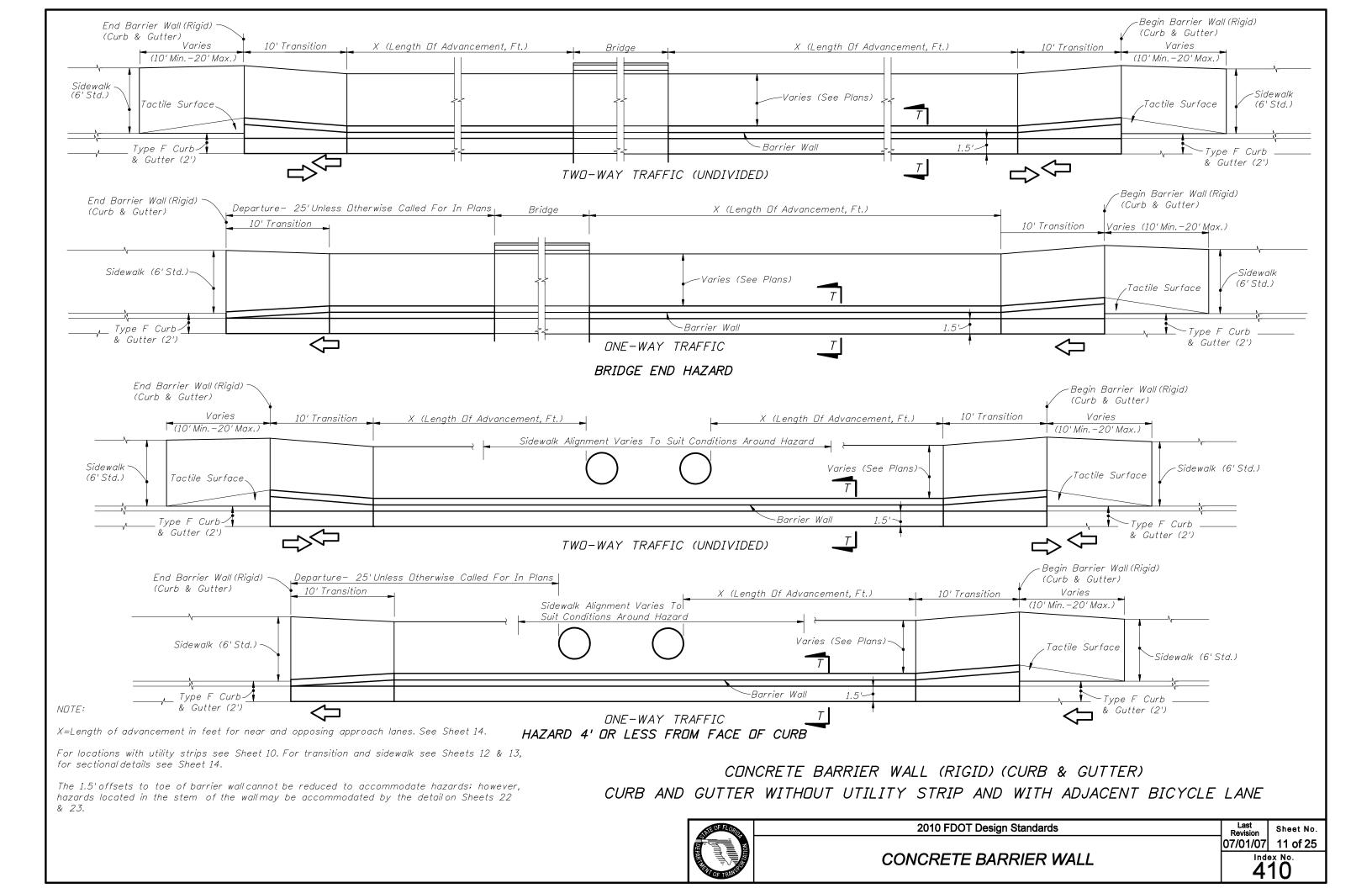
## REINFORCED CONCRETE BARRIER WALL (RETAINING)

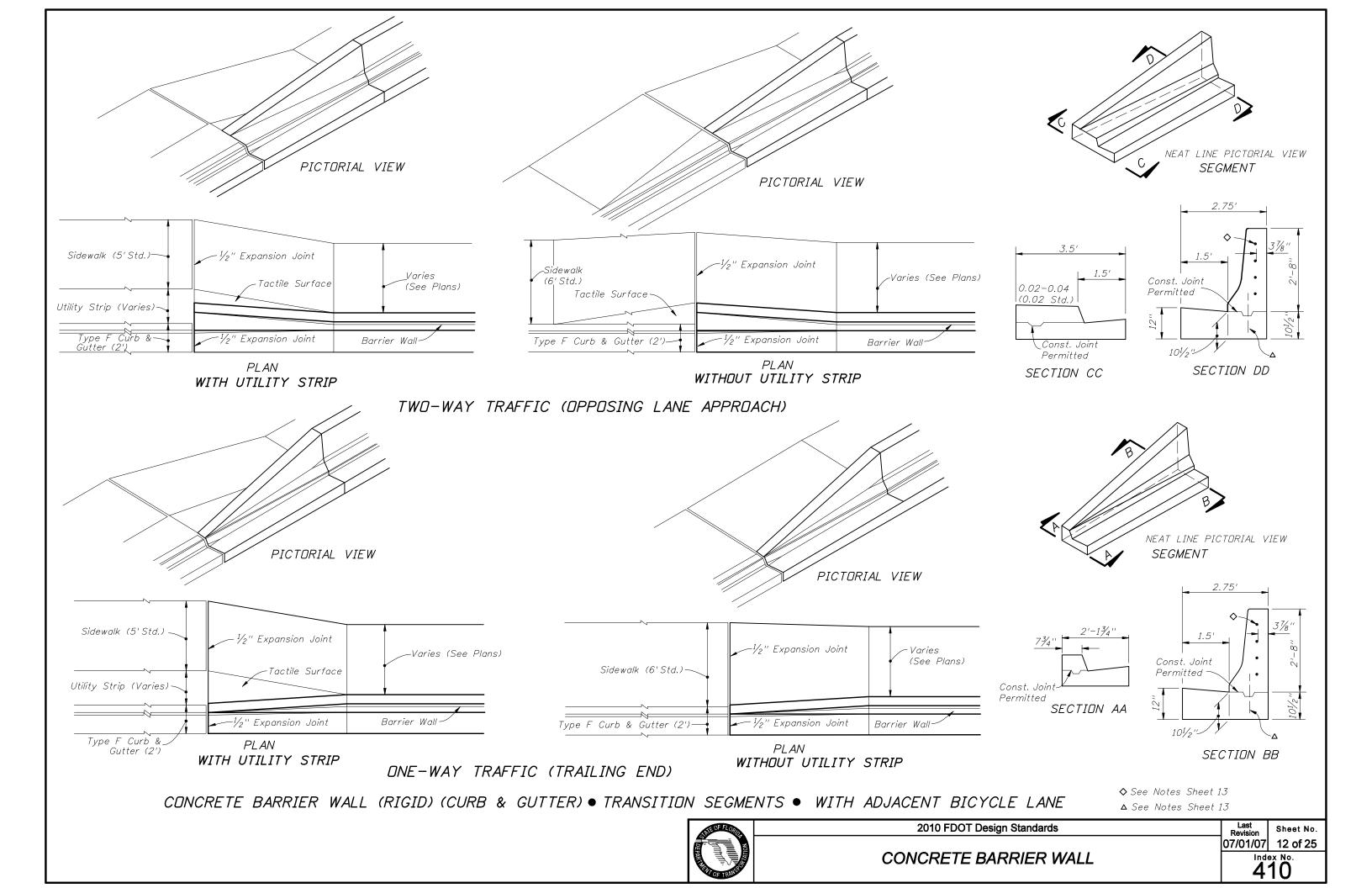


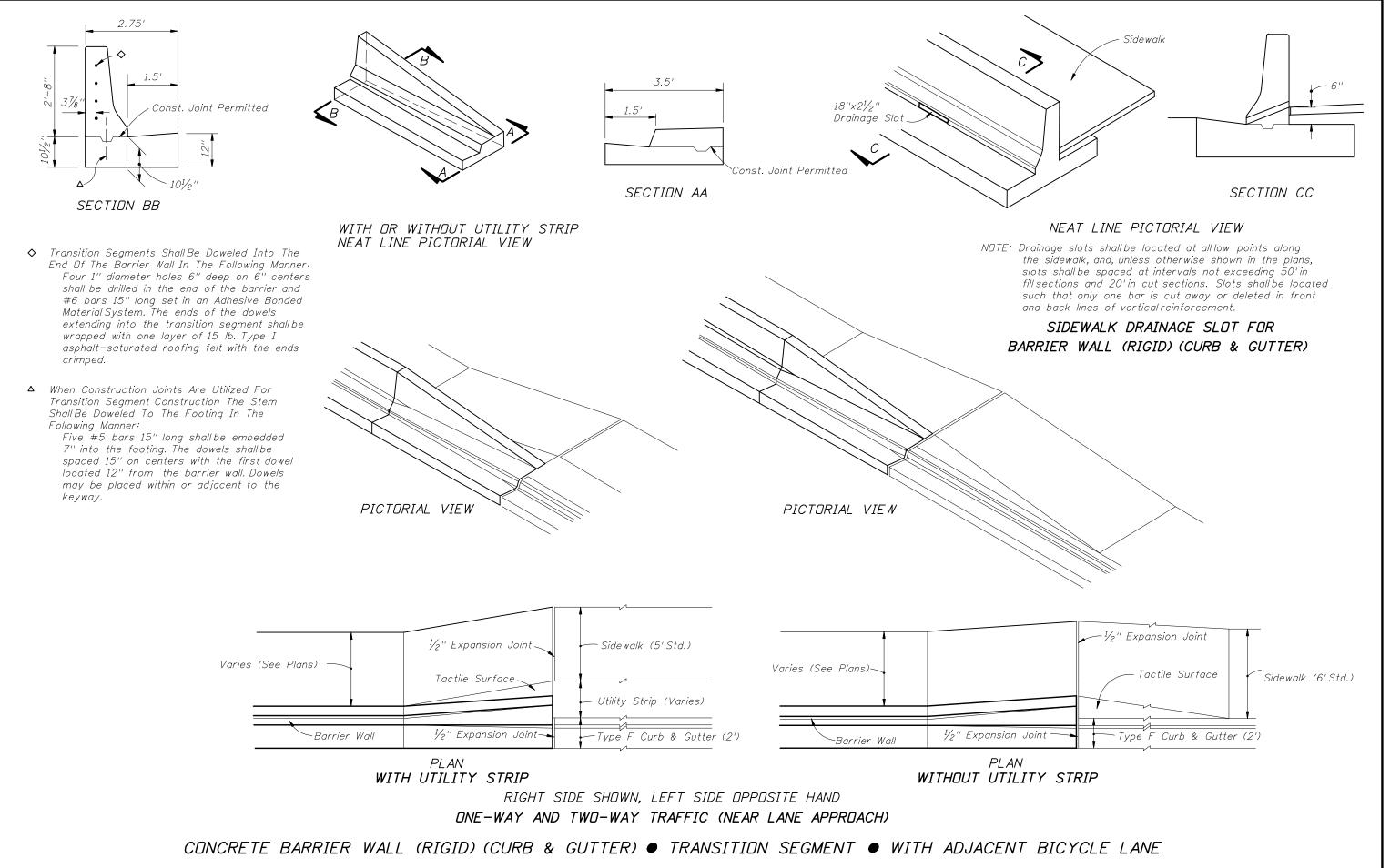
2010 FDOT	Design	Standards
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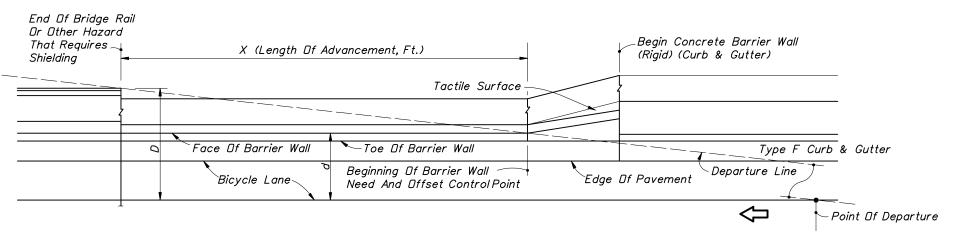




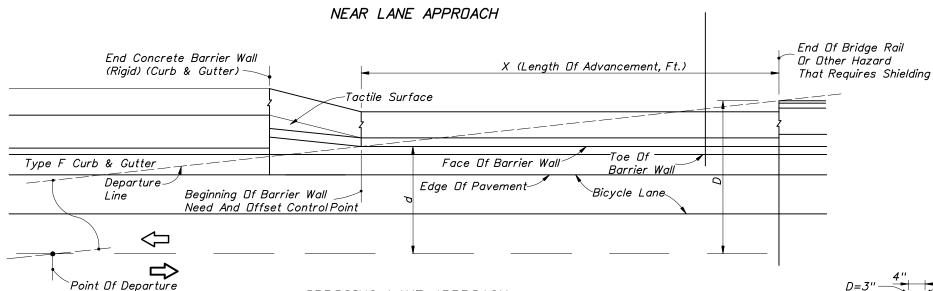
2010 FDOT Design Standards

CONCRETE BARRIER WALL

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### RIGHT SIDE APPROACH SHOWN - LEFT SIDE OPPOSITE HAND



### OPPOSING LANE APPROACH

WITH OR WITHOUT UTILITY STRIP - UTILITY STRIP SHOWN - SEE SHEET 10 & 11 FOR APPLICATIONS

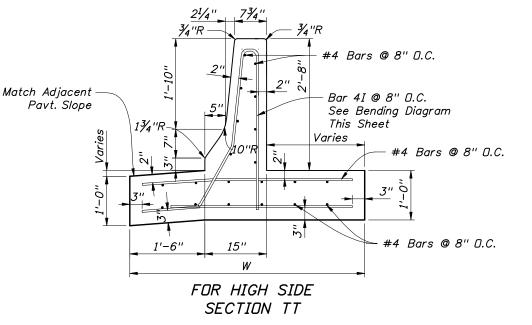
Design Speed mph	Length Of Advancement, Ft. (X)
≤45	=16 (D-d)
	The minimum length of advancement for ear and opposing lane approaches is 40'.

### Equation Variables:

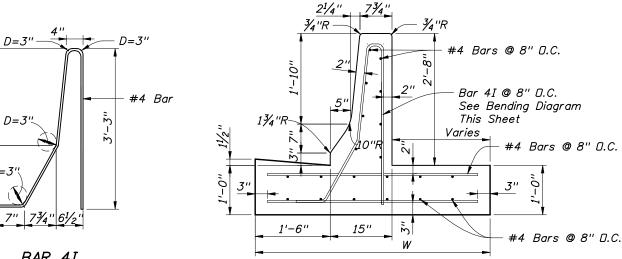
- D= Distance in feet from near edge of the near approach traffic lane to back of hazard or clear zone width whichever is lesser. For left side hazards and clear zones on two-way undivided facilities D is measured from the inside edge of the near approach traffic lane.
- d= Distance in feet from near edge of the near approach traffic lane to the face of barrier (at offset control point). For left side hazards on two-way undivided facilities d is measured from the inside edge of the nearest opposing traffic lane.

### LENGTH OF ADVANCEMENT

CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) WITH ADJACENT BICYCLE LANE



QUANTITIES						
Length∦ Of Barrier Wall	W	Class II Conc. CY Per Lin. Ft.	Rein. Steel Lbs. Per Lin. Ft.			
>73'	4'-9"	0.26	29			
56' to 73'	5'-6"	0.29	32			
48' to 55'	6'-0"	0.31	34			
41' to 47'	6'-6"	0.33	<i>37</i>			
36' to 41'	7'-0"	0.35	39			
29' to 35'	8'-0"	0.38	42			



BAR 4I BENDING DIAGRAM

D=3"

D=3'

SECTION TT

FOR LOW SIDE

Note: All longitudinal reinforcement #4 bars. Shorter segments due to construction or expansion joint shall be doweled in the manner described for 'Transition Segments' on Sheet 13.

Transverse expansion joints are to be constructed at the juncture of wall transitions and curb and gutter, and at intervals so that spacing will not exceed 100'.

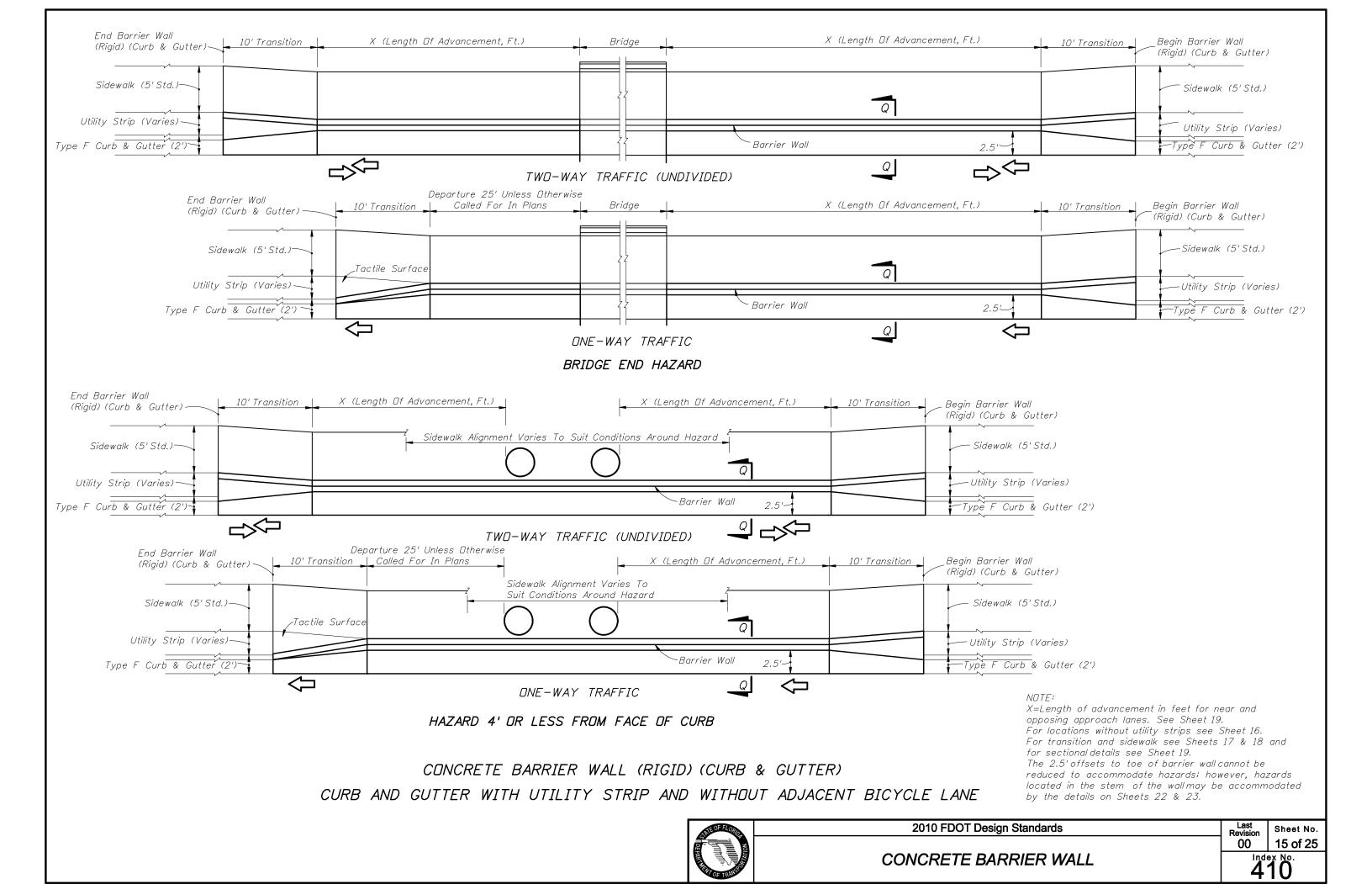
For barrier wall inlet details see Index No. 219. Inlet extends into bicycle lane 12". Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Curb & Gutter), LF.

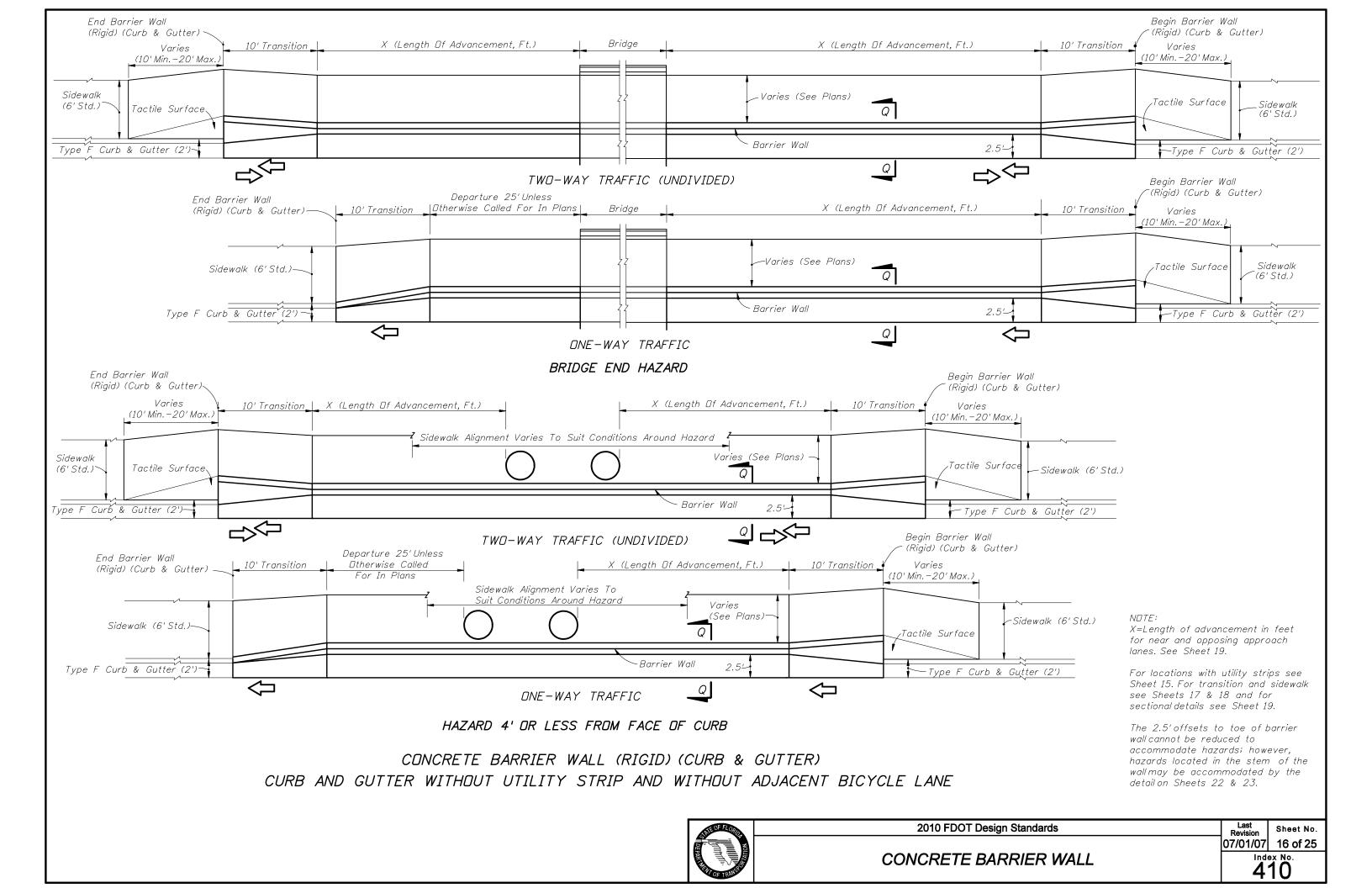


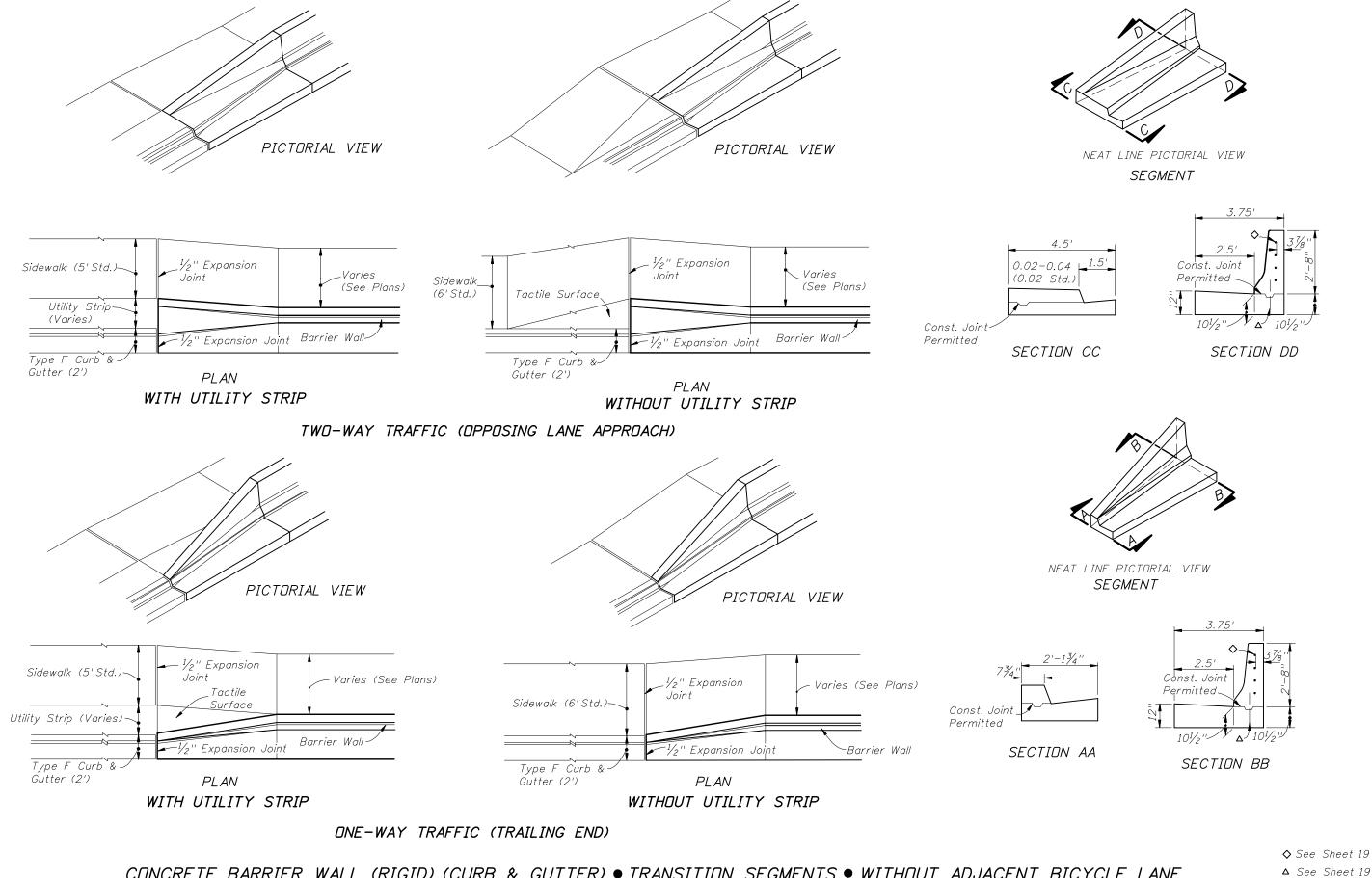
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1ndex No.





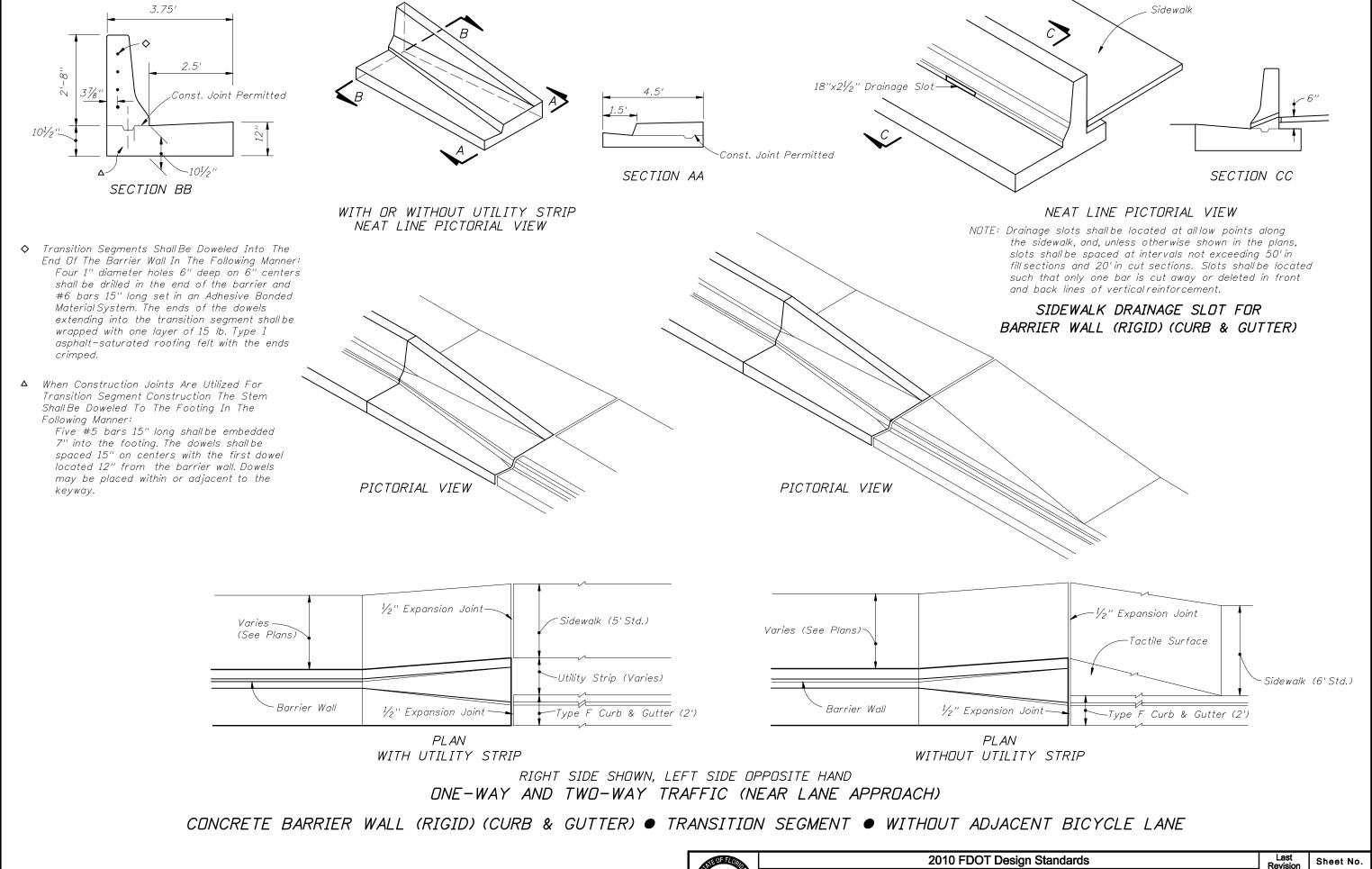


CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) ● TRANSITION SEGMENTS ● WITHOUT ADJACENT BICYCLE LANE



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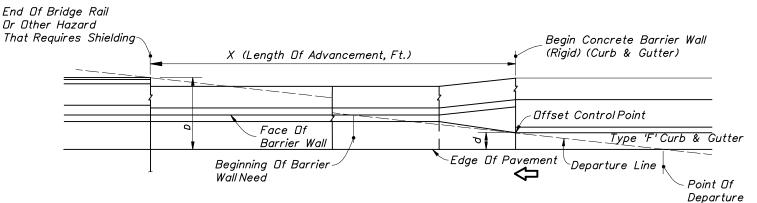
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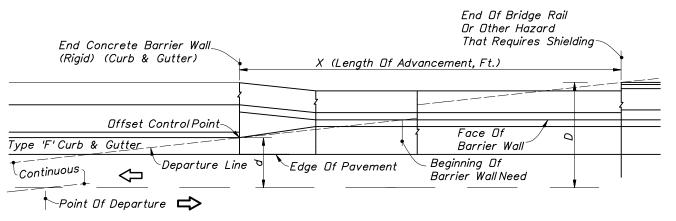
CONCRETE BARRIER WALL

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RIGHT SIDE APPROACH SHOWN - LEFT SIDE OPPOSITE HAND

NEAR LANE APPROACH



### OPPOSING LANE APPROACH

WITH OR WITHOUT UTILITY STRIP - UTILITY STRIP SHOWN SEE SHEET 15 & 16 FOR APPLICATIONS

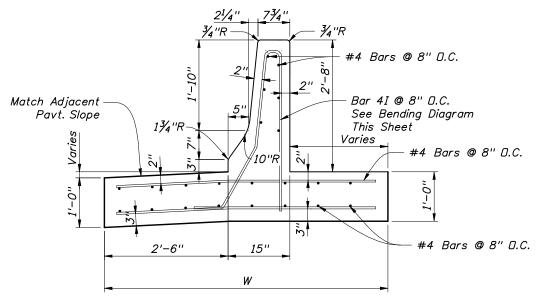
Design Speed mph	Length Of Advancement, Ft.(X)
≤ <b>4</b> 5	16 (D-d)

Note: The minimum length of advancement for both near and opposing lane approaches is 40'.

#### Equation Variables:

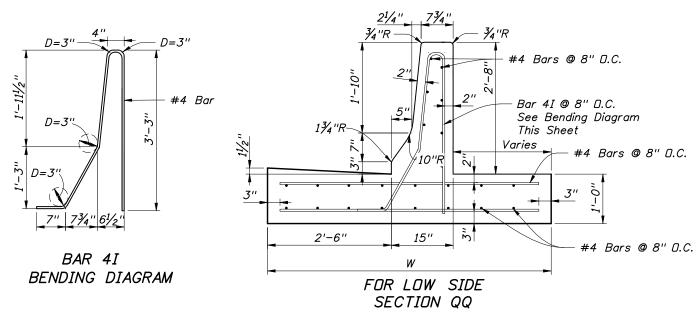
- D= Distance in feet from near edge of the near approach traffic lane to back of hazard or clear zone width whichever is lesser. For left side hazards and clear zones on two-way undivided facilities D is measured from the inside edge of the near approach traffic lane.
- d= Distance in feet from near edge of the near approach traffic lane to the face of curb (at offset control point). For left side hazards on two-way undivided facilities d is measured from the inside edge of the nearest opposing traffic lane.

LENGTH OF ADVANCEMENT



QUANTITIES					
Length* Of Barrier Wall	W	Class II Conc. CY Per Lin. Ft.	Rein. Steel Lbs. Per Lin. Ft.		
<i>&gt;73'</i>	4'-9"	0.26	29		
56' to 73'	5'-6"	0.29	32		
48' to 55'	6'-0"	0.31	34		
41' to 47'	6'-6"	0.33	<i>37</i>		
36' to 41'	7'-0"	0.35	39		
29' to 35'	8'-0"	0.38	42		

FOR HIGH SIDE SECTION QQ



Note: All longitudinal reinforcement #4 bars. Shorter segments due to construction or expansion joint shall be dowled in the manner described for 'Transition Segments' on Sheet 18.

Transverse expansion joints are to be constructed at the juncture of wall transitions and curb and gutter, and at intervals so that spacing will not exceed 100'.

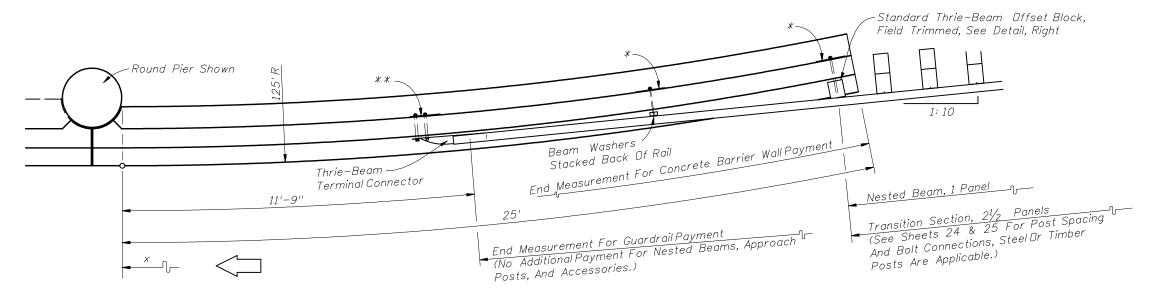
For barrier wall inlet details see Index No. 219. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Curb & Gutter), LF.

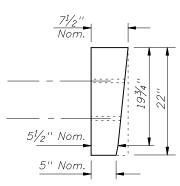
Sheet No.

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## CONCRETE BARRIER WALL (RIGID) (CURB & GUTTER) ● WITHOUT ADJACENT BICYCLE LANE





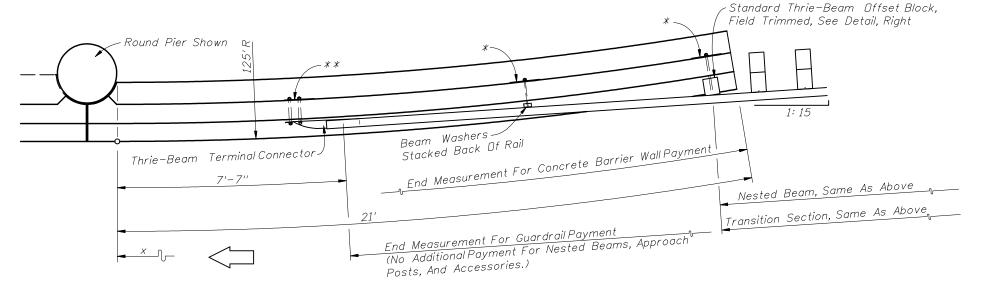


FOR USE WITH EITHER 1:10 OR 1:15 GUARDRAIL TRANSITIONS

STANDARD THRIE-BEAM OFFSET BLOCK (FIELD TRIMMED)

### PLAN FOR DESIGN SPEED ≤ 45 MPH

NOTE: For details at Rigid Hazard see Sheet 21.



### PLAN FOR DESIGN SPEED ≥ 50 MPH

Note: For continuous barrier between independent bents or single pier columns see Sheets 21-23.

ARC LENGTH (FT)	DISTANCE "x" (FT)	OFFSETS "y" (FT)	125' R
4	4.00	0.06	
8	7.99	0.26	Y
12	11.98	0.58	X
16	15.96	1.02	Note:
20	19.91	1.60	Wall may be constructed in chords having lengths
21	20.91	1.76	<pre></pre>
24	23.85	2.30	<u> </u>
25	24.83	2.49	

SHOULDER BARRIER WALL AT ABOVE GROUND RIGID HAZARDS
WHEN GUARDRAIL OFFSET FROM HAZARD LESS THAN 3'

### NOTES

- 1. This wall is intended for use where the wall has bearing against the hazard; when the length between bent supports or pier columns exceeds 13', the affected segments shall be constructed in accordance with the detail for 'Reinforced Concrete Barrier Wall (Shoulder)', 'Section TT' or 'Section QQ', this index.

  In cases where the barrier wall and slope pavement or other structure would occupy the same location, the wall and structure are to be modified as detailed in the plans.
- 2. The barrier wall radial segments are intended for use on approach and trailing ends of both one-way and two-way facilities. The guardrail connections shown on this sheet apply to one-way approaches and to the approaching and trailing ends of two-lane two-way facilities. On trailing ends of two-way multilane and one-way facilities the end connection on Sheet 1 may be used.

For walls with normal offsets from hazards and their guardrail connections, see Sheet 24 & 25.

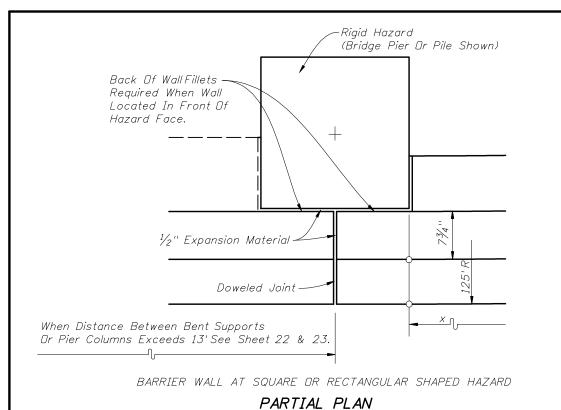
- 3. Refer to Index No. 400 for additional guardrail information.
- 4. Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder), LF.
- \*  $12"x12"x\frac{1}{4}"$  galvanized steelback-up plate with  $\frac{5}{6}$ " post bolts (either 14" or 18" long) and nuts with  $\frac{5}{6}$ " plain round washers under nuts.
- \*\* Attach thrie-beam terminal connector to shoulder barrier wall with a  $21"x12"x^{5}_{8}"$  thrie beam terminal connector plate and  $5-\frac{7}{8}"x12"$  long HS hex bolts and nuts with  $\frac{7}{8}"$  plain round washers under heads and nuts.



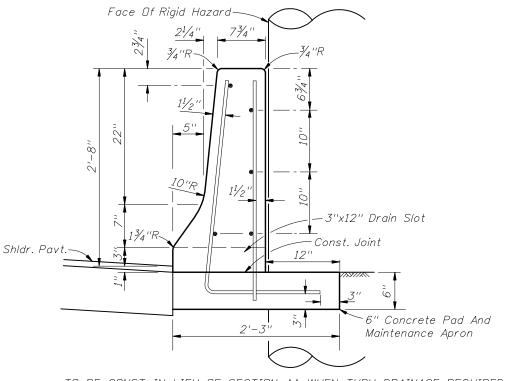
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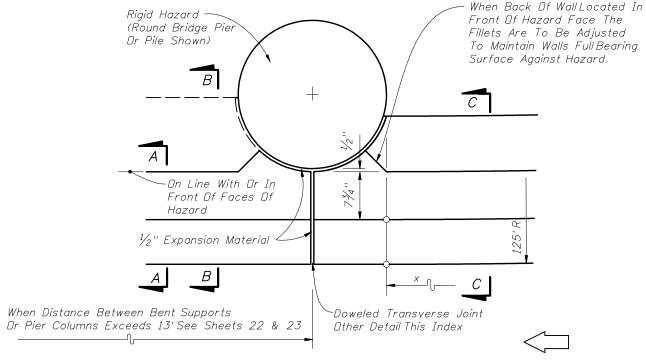
Face Of Rigid Hazard — Shldr. Pavt. -15''

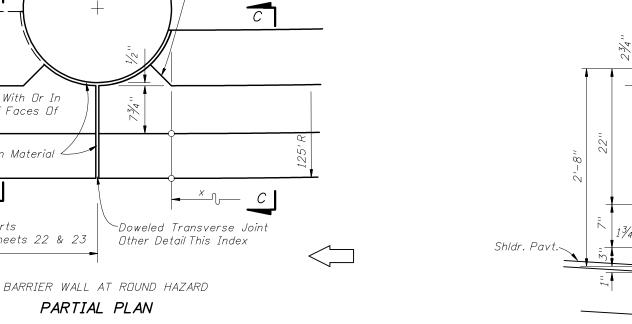


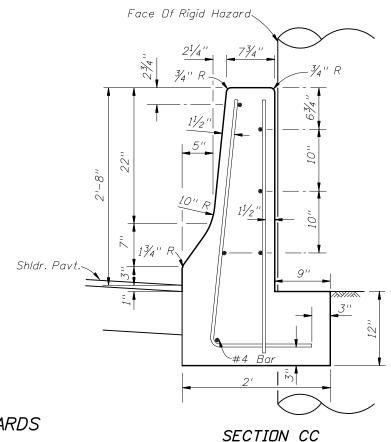
TO BE CONST. IN LIEU OF SECTION AA WHEN THRU DRAINAGE REQUIRED

### SECTION AA

SECTION BB

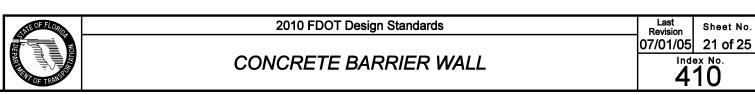


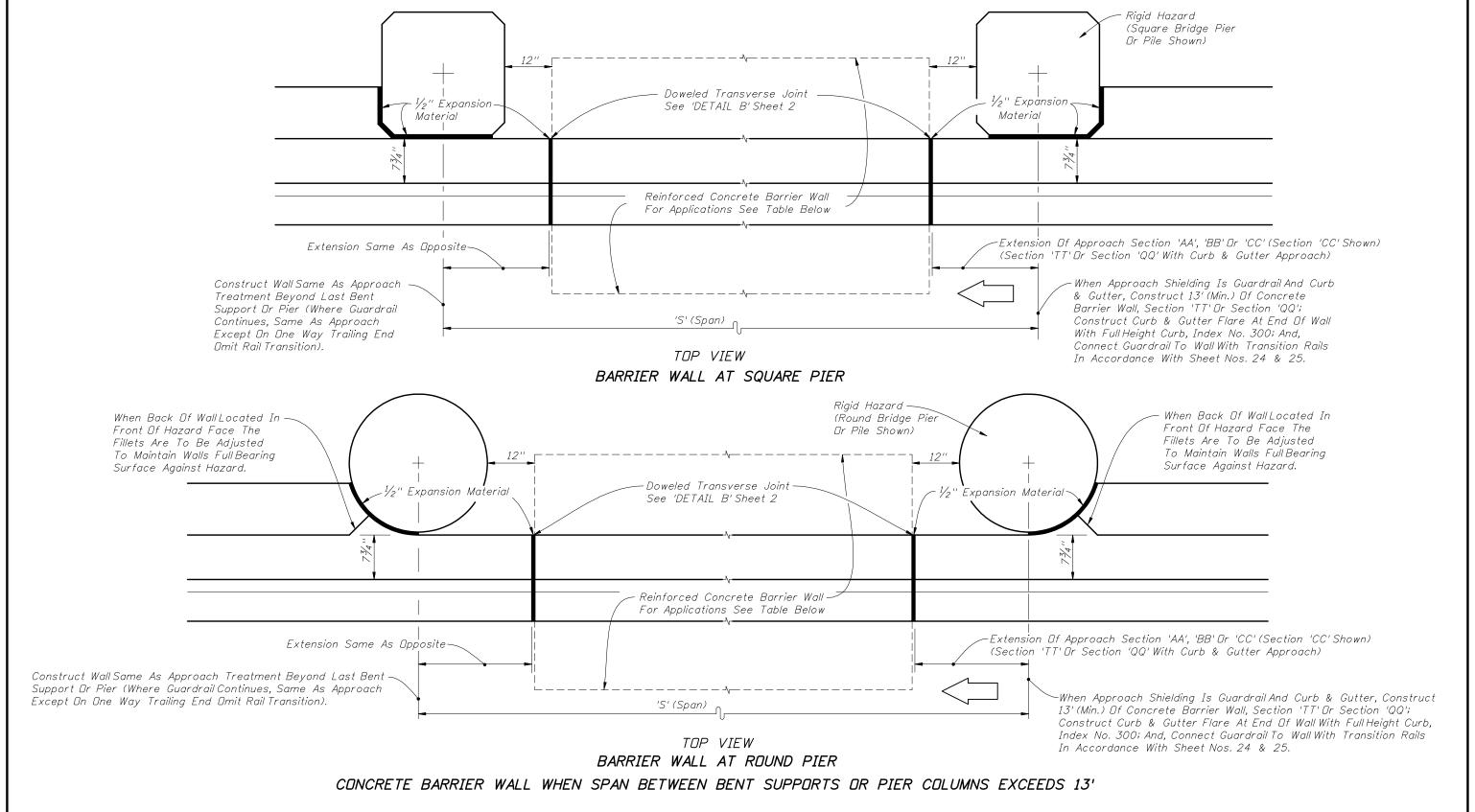




SHOULDER BARRIER WALL AT ABOVE GROUND RIGID HAZARDS WHEN GUARDRAIL OFFSET FROM HAZARD LESS THAN 3'

All vertical reinforcement #4 bars at 12" centers. All horizontal reinforcement #5 bars.





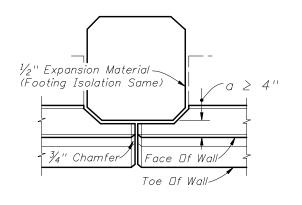
'S' Feet REINFORCED CONCRETE BARRIER WALL APPLICATIONS

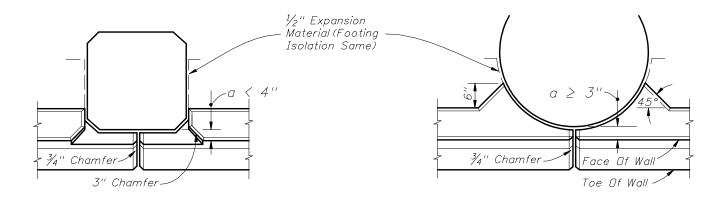
>13' Reinforced Concrete Barrier Wall (Shoulder)' With Flush Shoulders; Or, Section 'TT' Or Section 'QQ' With Curb & Gutter

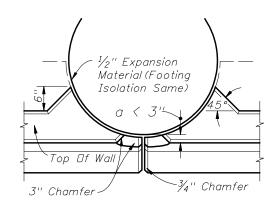
Barrier wall footings that conflict with bent or pier foundations shall be modified as described in the plans.

CONCRETE BARRIER WALL WHEN GUARDRAIL OFFSET FROM BENT OR PIER LESS
THAN 3 FEET OR WHERE WALL STEM ABUTS SUPPORTS OR PIER COLUMN

INTE OF FLORID	2010 FDOT Design Standards	Last Revision	Sheet No.
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STATION STATION	CONCRETE BARRIER WALL	Inde	No.
OF TRANS!		4	<u>10</u>







TOP VIEWS

'a' Varies (Circular Or Octagonal Hazard Not More Than 2" In Front Of Face Of Wall). Applicable To Sections 'AA' And 'BB' With Spans Of ≤ 13', And To Section 'CC', Sheet No. 21. Applicable To Other Rigid Walls Of This Index For Spans > 13' Unless Otherwise Shown In The Plans.

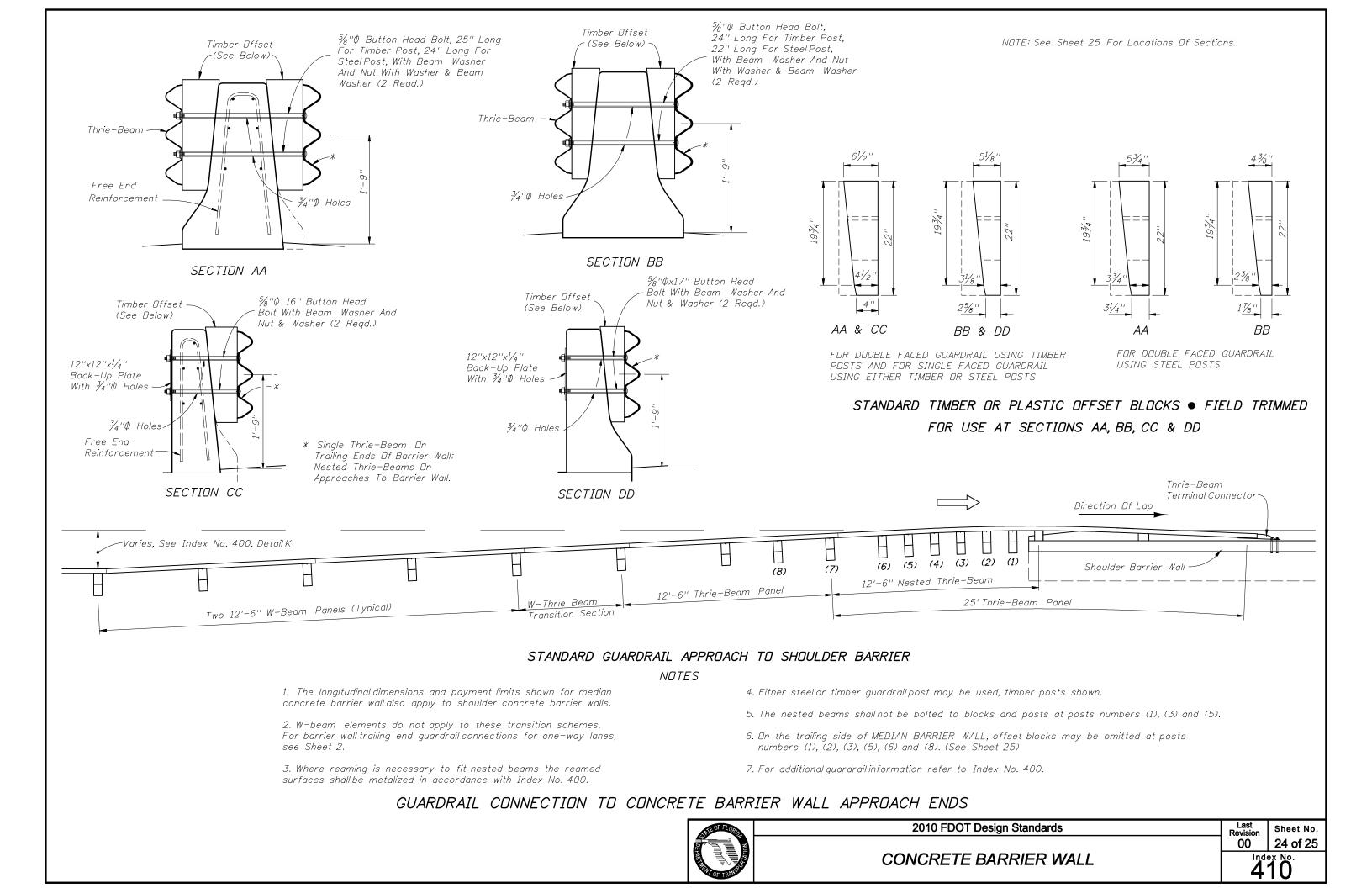
## HAZARD PENETRATING STEM OF RIGID CONCRETE BARRIER WALLS

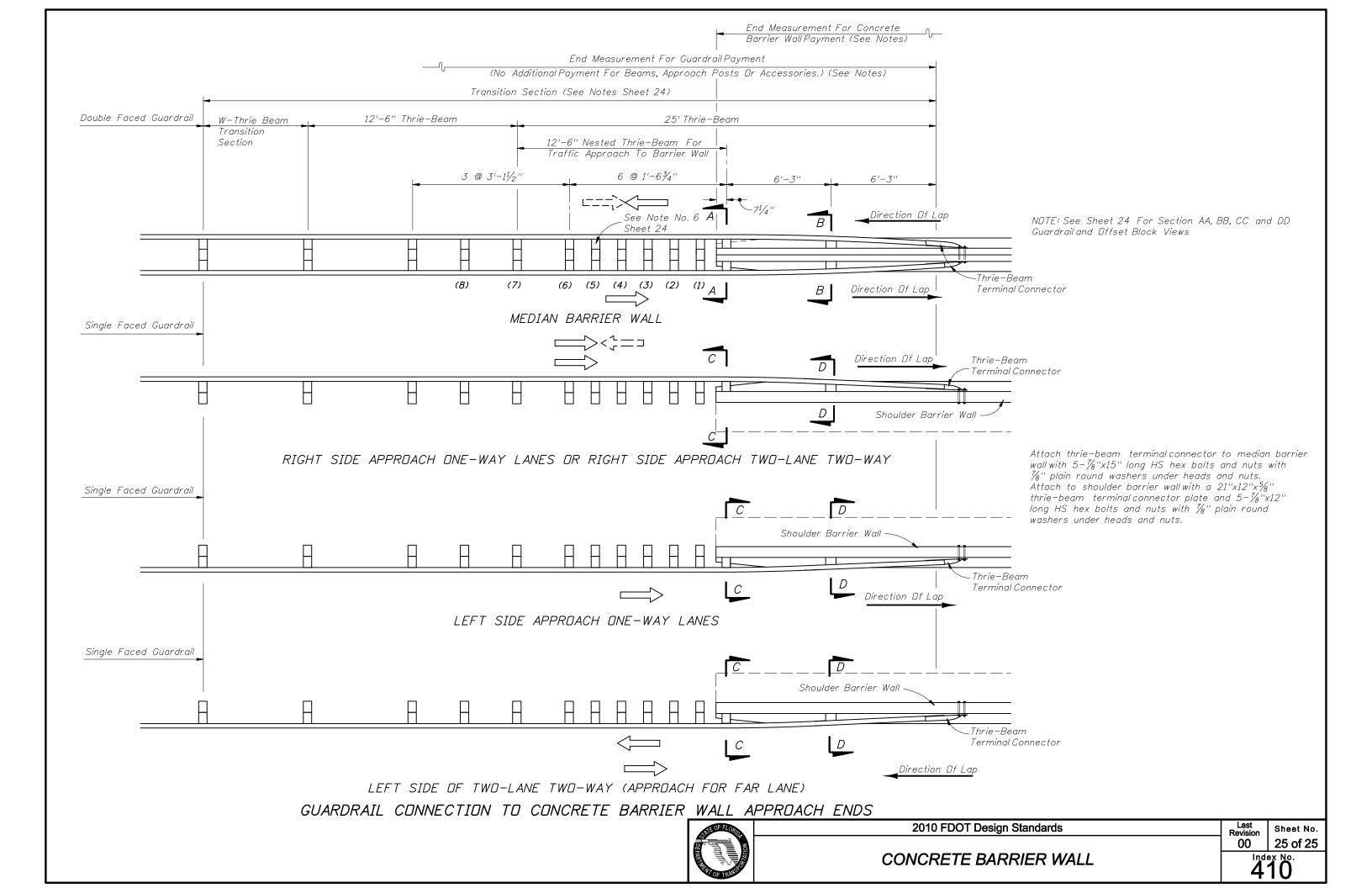
The details on sheets 22 & 23 are treatments to the F-shape concrete barrier walls depicted on Sheet Nos. 9 through 19, where site conditions impose reduced clearances between above ground hazards and the walls. Bridge bent supports and piers are shown. These treatments are not applicable to hazards that cannot provide lateral support for the walls. See the plans for limits of wall sections applied and other associated wall treatments.



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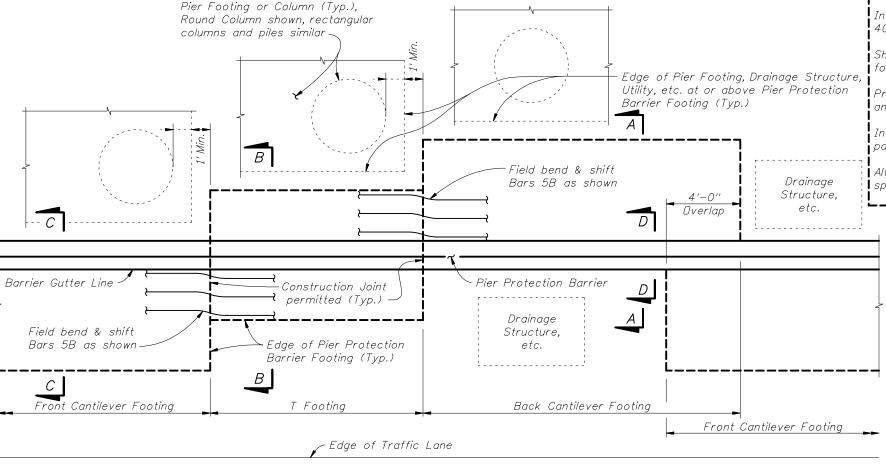




This Pier Protection Barrier has been structurally evaluated to be equivalent or greater in strength to other safety shape traffic barriers which have been crash tested to NCHRP Report 350 TL-5 criteria. This barrier meets the requirements of the AASHTO LRFD Bridge Design Specifications for a barrier used for bridge pier protection.

### GENERAL NOTES:

- 1. Concrete shall be Class III or IV unless otherwise called for in the plans. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Specifications, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finished Coating in accordance with Section 400 only when called for in the plans.
- 2. Construct Pier Protection Barrier continuous without transverse contraction or expansion joints. Transverse construction joints may be used at a spacing greater than or equal to 40'. Provide longitudinal reinforcing steel continuous across construction joints.
- 3. When the Pier Protection Barrier is installed adjacent to Roadway or Shouler pavement, compact the top 12'' of the subgrade to at least 100% of the density as defined in the AASHTO T-99 specifications.
- 4. Isolate Barrier Wall Inlets, Index 218, from Pier Protection Barriers and Footings with 1" expansion material.
- 5. On roadways designated for reverse laning, mark all downstream barrier ends that are not shielded or outside the clear zone with Type 3 Object Markers. Include the cost of the Object Marker in the cost of the Pier Protection Barrier.
- 6. Payment: Pier Protection Barrier and Crash Wall to be paid for under the contract unit price for Shoulder Concrete Barrier Wall (Rigid-Shoulder 42"), LF, or Shoulder Concrete Barrier Wall (Rigid-Shoulder 54"), LF.



PLAN VIEW

INSTRUCTIONS TO DESIGNER:

As used in this standard, setback distance is as defined by LRFD. See PPM and Index 700 for minimum recoverable terrain and horizontal clearance requirements.

Establish the offset from the Pier Protection Barrier to the bridge pier, column or pile bent based on project constraints.

Determine the required Pier Protection Barrier height, i.e. 42" or 54", in accordance with the requirements of the LRFD Bridge Design Specifications and the Structures Design Guidelines.

Determine the appropriate limiting stations of the Pier Protection Barrier and its end treatment(s) using the Pier Protection Barrier Length of Advancement diagrams provided.

Select Pier Protection Barrier terminal treatment for design speeds greater than or equal to 50 mph:

- a. Terminated outside of the clear zone of any approach traffic; b. Terminated within a shielded location;
- o. Terminatea within a shleidea location,
- c. Terminal protection by the use of a crash cushion system; or,
- d. Terminated in conjunction with a suitably designed transition to another barrier.

Determine the appropriate footing configuration(s) (T, Front Cantilever or Back Cantilever) for a continuous run of Pier Protection Barrier using the Pier Protection Barrier Footing Layout Schematics. Select the footing configuration(s) based on traffic control needs and locations of piers, pier footings, utilities, drainage structures, etc. as shown. Footing configurations along a continuous run of Pier Protection Barrier may be intermixed as shown.

Designate the Pier Protection Barrier height, footing configuration(s) and limiting stations on the Plan-Profile sheets, e.g.:

Begin 42" Pier Protection Barrier with Front Cantilever Footing, Sta. 100+00.00

Indicate Crash Wall locations (when required) and lengths on the Plan-Profile sheets. Designate Crash Wall height to match height of adjacent Pier Protection Barrier.

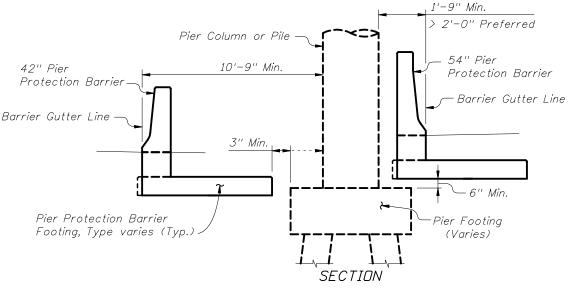
In absence of continuous concrete barrier, determine guardrail requirements in accordance with Indexes 400 and 410.

Show Cross Sections as required to locate Pier Protection Barrier, Crash Wall (when required) and footings adjacent to bridge piers, columns or footings, drainage structures, utilities, etc.

Prepare Traffic Control Plans to accommodate Pier Protection Barrier, Crash Wall (when required) and footing construction.

Include length(s) of Crash Walls (measured along front face) in length of Pier Protection Barrier for payment.

Although intended for shielding bridge piers, the Pier Protection Barrier can be used on a project specific basis to shield other critical roadside objects when deemed necessary or appropriate.

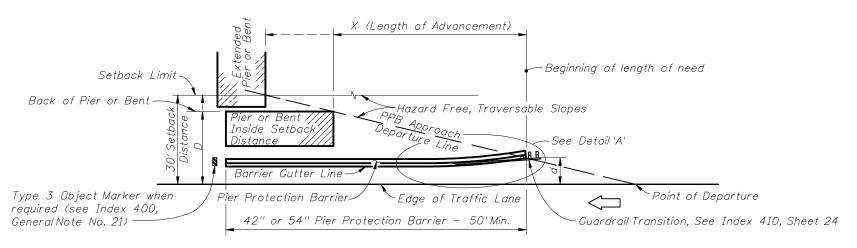


PIER PROTECTION BARRIER FOOTING LAYOUT SCHEMATICS

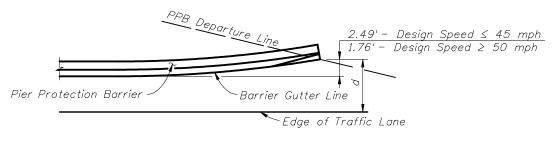


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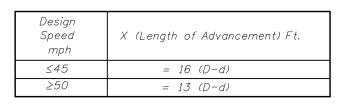


## (LEFT SIDE OPPOSITE HAND) ONE-WAY TRAFFIC



DETAIL 'A'

(Guardrail not shown for clarity)

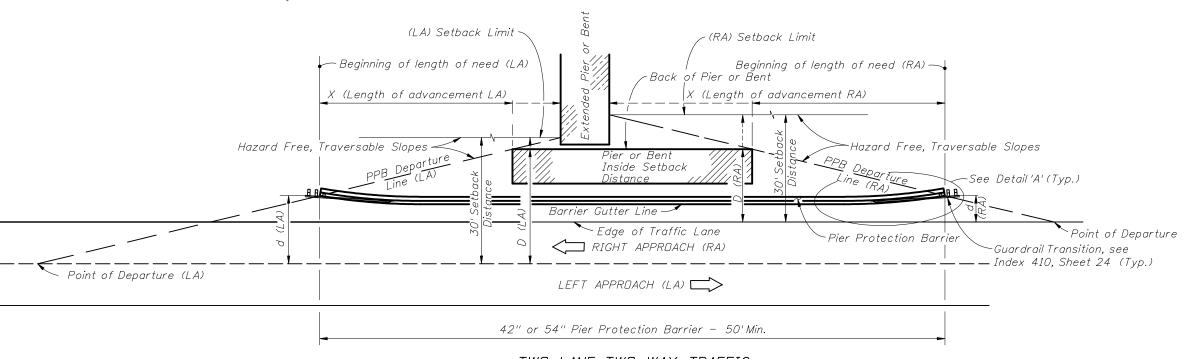


### NOTE:

Length of Advancement determined from the diagrams and equations shown establishes the location of the upstream beginning length of need for a Pier Protection Barrier, however, the Length of Advancement for the combination of Pier Protection Barrier and required guardrail can be no less than that required by other details of Index 400.

### Equation Variables:

- D=D Distance in feet from the near edge of the near approach traffic lane to either (a) the back of pier, when the pier is located inside the Setback Distance or (b) the Setback Distance, when the pier extends to or goes beyond the Setback Distance. For left side piers on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane.
- d = Distance in feet from the near edge of the near approach traffic lane to the Pier Protection Barrier gutter line at its intersection with the departure line or the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane.



TWO-LANE TWO-WAY TRAFFIC

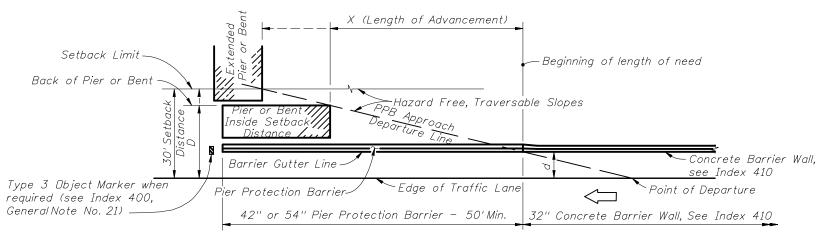
NOTE: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams.

PPB = Pier Protection Barrier

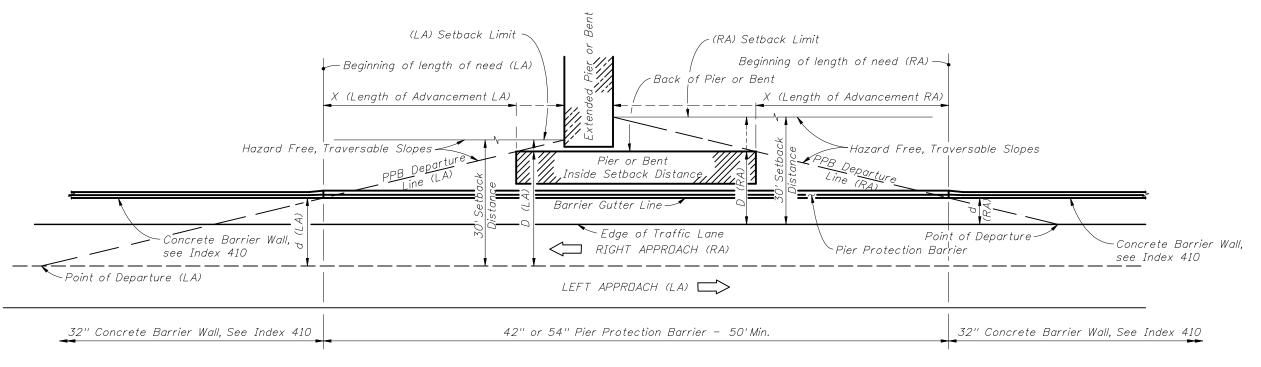
## LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH GUARDRAIL CONTINUATION



Index No. 411



## (LEFT SIDE OPPOSITE HAND) ONE-WAY TRAFFIC

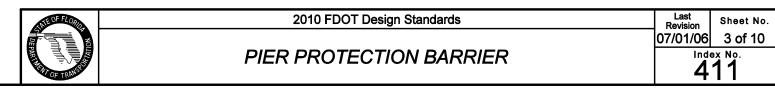


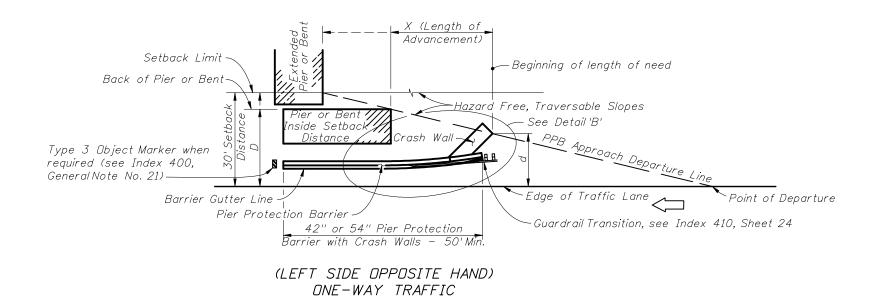
### TWO-LANE TWO-WAY TRAFFIC

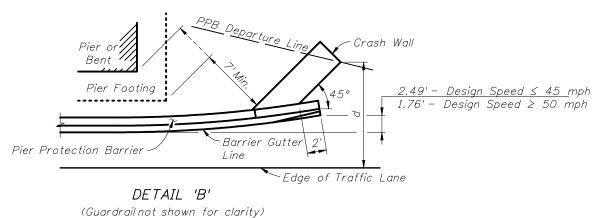
NOTES: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams. PPB = Pier Protection Barrier

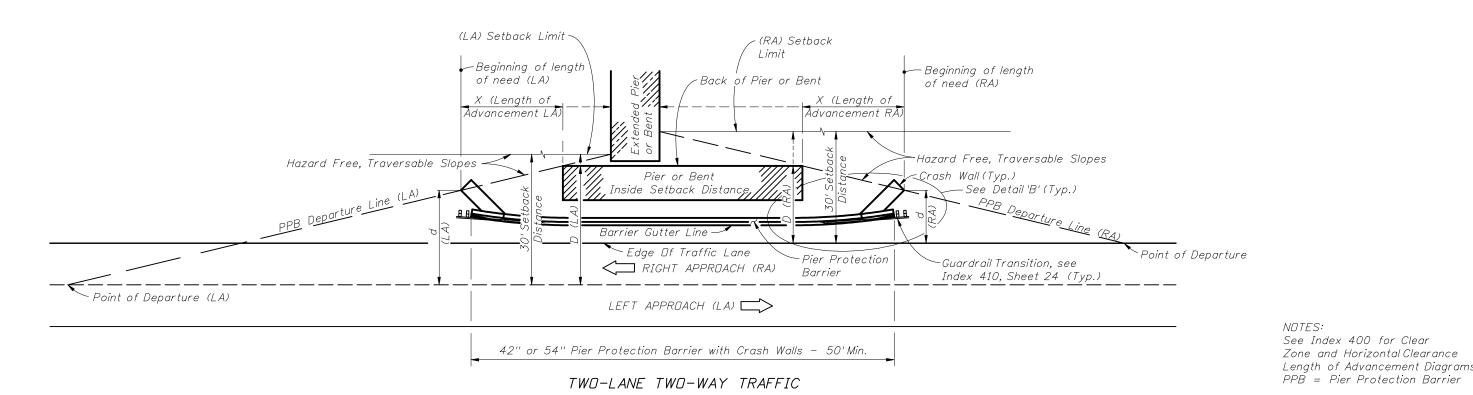
See Notes on Sheet 2.

## LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH CONCRETE BARRIER WALL CONTINUATION

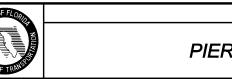






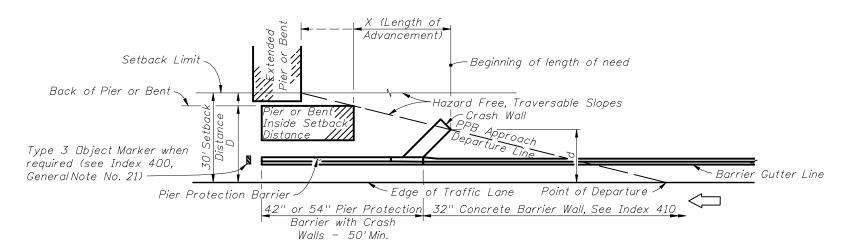


LENGTH OF ADVANCEMENT DIAGRAMS - PIER PROTECTION BARRIER WITH CRASH WALL AND GUARDRAIL CONTINUATION

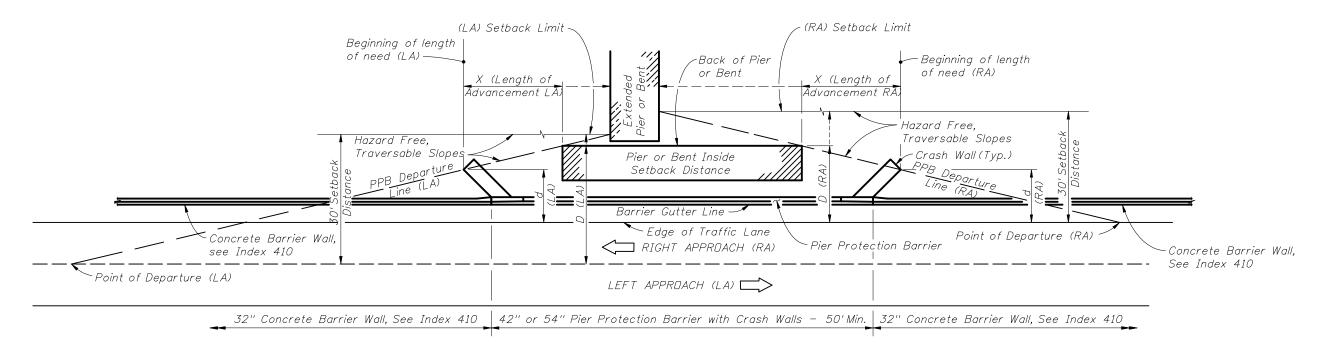


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R PROTECTION BARRIER		ex No.

See Notes on Sheet 2.



## (LEFT SIDE OPPOSITE HAND) ONE-WAY TRAFFIC

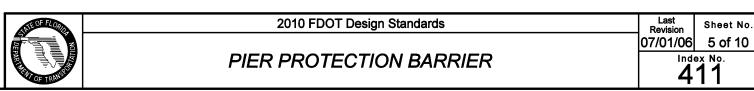


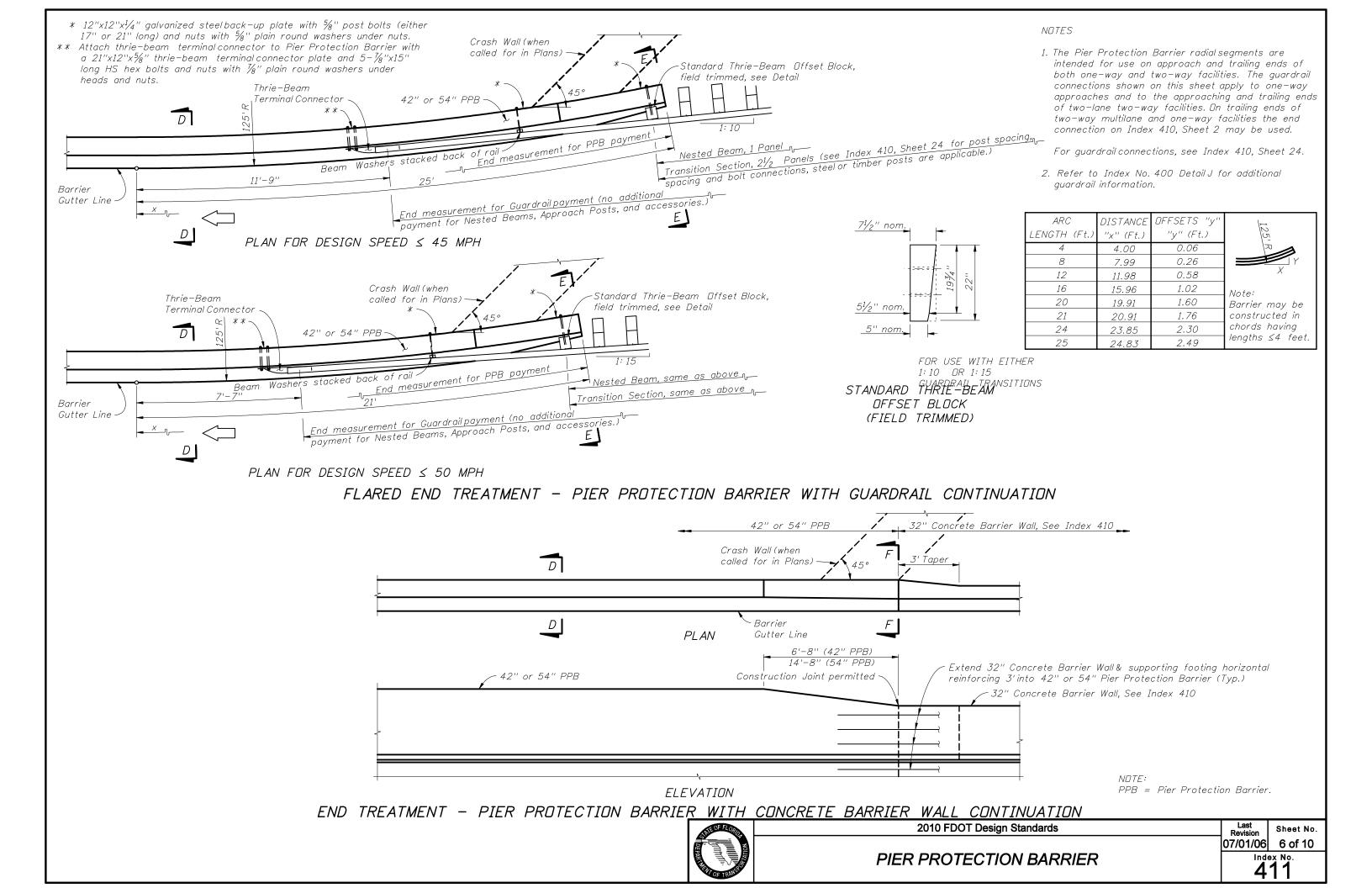
TWO-LANE TWO-WAY TRAFFIC

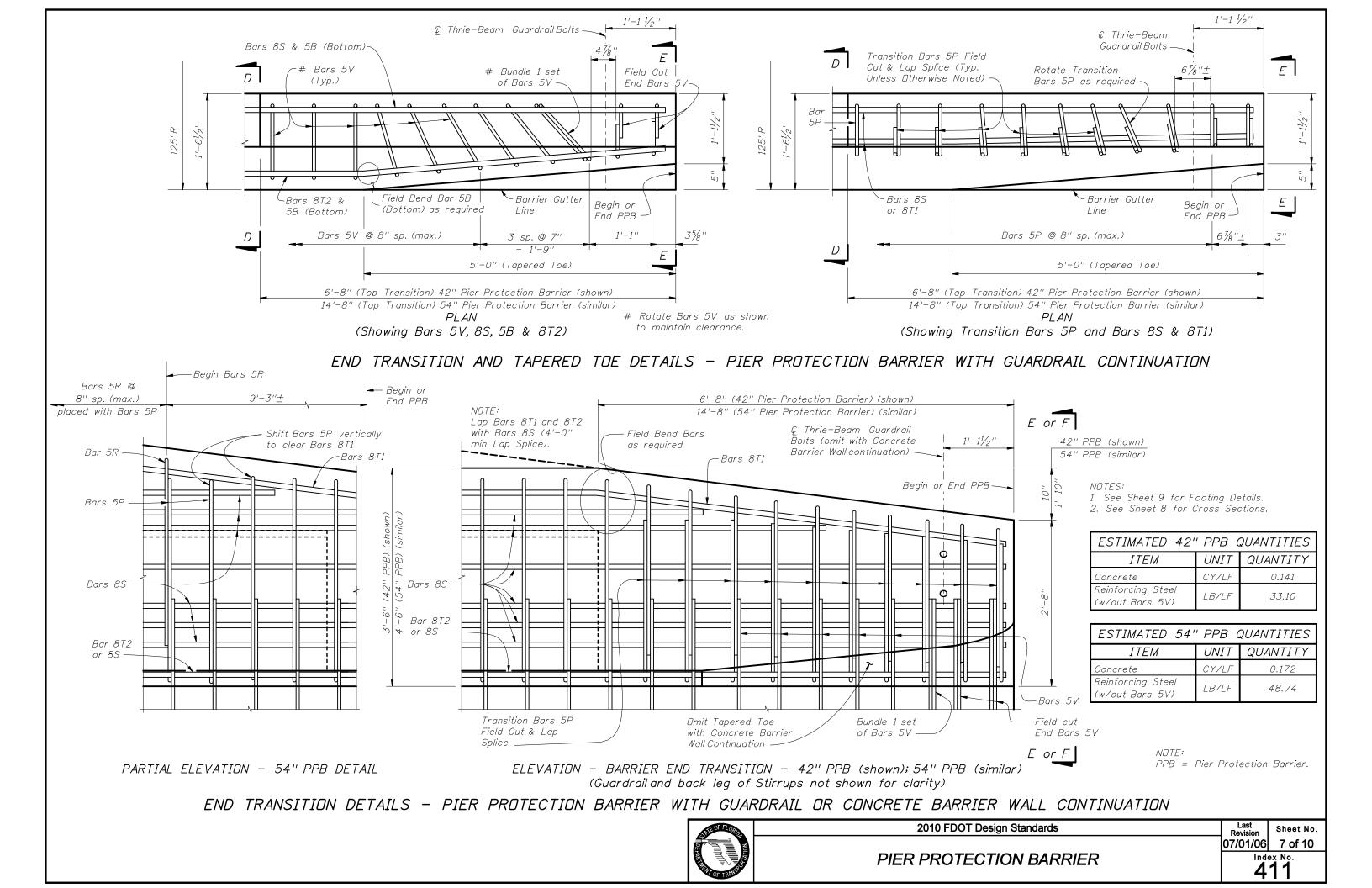
NOTES: See Index 400 for Clear Zone and Horizontal Clearance Length of Advancement Diagrams. PPB = Pier Protection Barrier

See Notes on Sheet 2.

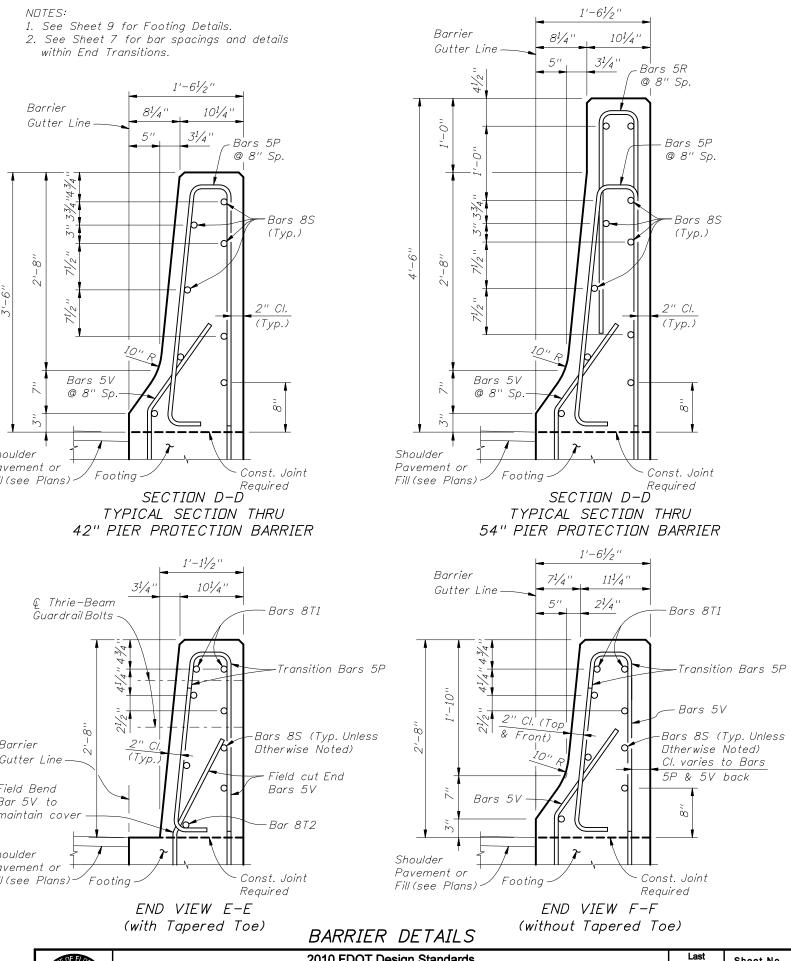
## LENGTH OF ADVANCEMENT DIAGRAMS — PIER PROTECTION BARRIER WITH CRASH WALL AND CONCRETE BARRIER WALL CONTINUATION



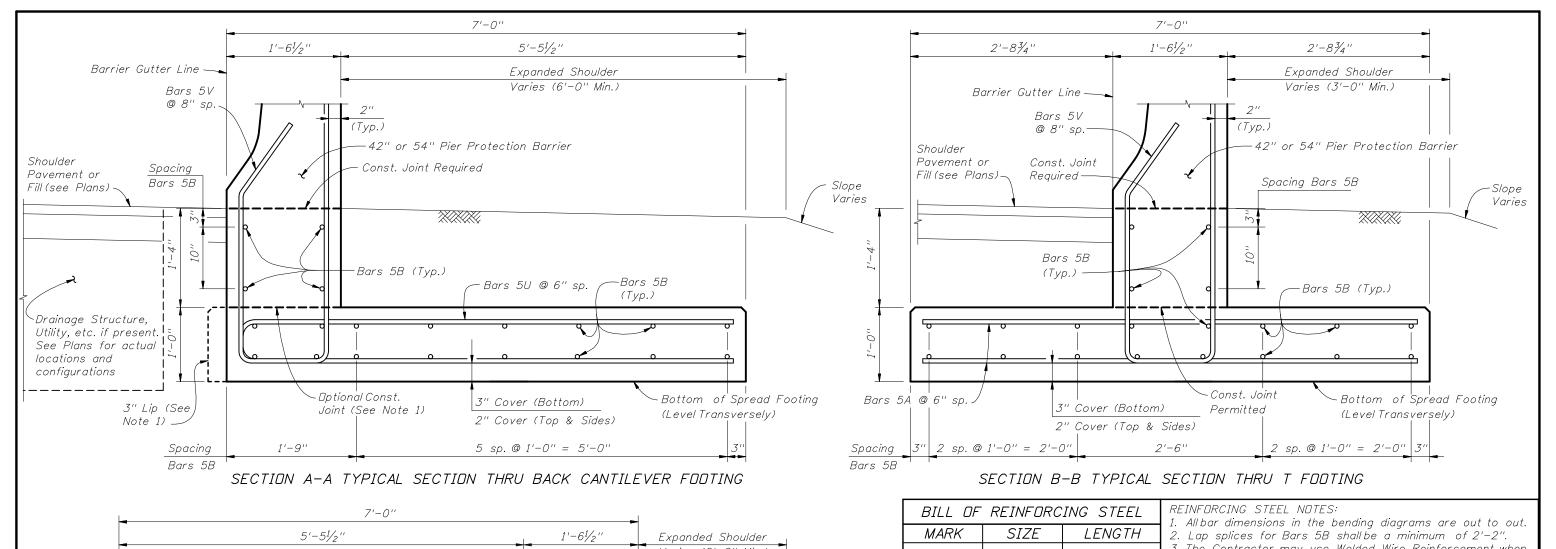


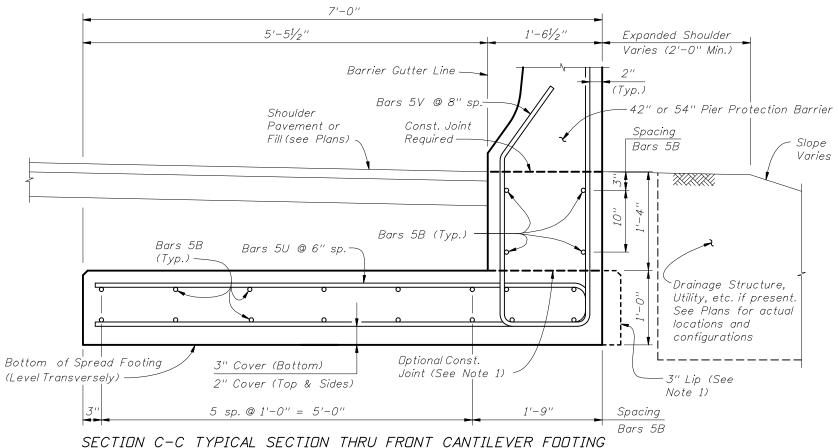


BILL OF REINF	DRCING STEEL	REINFORCING STEEL NOTES: 1. All bar dimensions in the bending diagrams are out to out.	Λ.
MARK	SIZE LENGTH	2. Bars 8S may be continuous or spliced at the construction	Λ 1: 2
<u> </u>	5 7'-6"	joints. Lap splices for Bars 8S shallbe a minimum of 4'-0''.  3. The Contractor may utilize Welded Wire Reinforcement when	2
<u></u>	5 6'-7" 8 As Regd.	approved by the Éngineer. Welded Wire Reinforcement shall	
42" PPB T1 & T2	8 13'-0"	conform to ASTM A497.	
54" PPB T1 & T2	8 21'-0"	NOTE: PPB = Pier Protection Barrier	E
V	5 9'-2"		(
	CONVENTIONAL	REINFORCING STEEL BENDING DIAGRAMS	
-	Length as Required		
		## Vary Field Cut from 7" to 1'-0"   Š to maintain a 2'-0" min. lap.	
	DAKS 03	$\begin{array}{c} 12 \\ 10 \\ 10 \end{array}$	
	Length as required	# Varies  (Field Cut in cover, 1C	
(within	BARS 8S Flared End Trea	# : Field Cut solution	3'-6''
	<del> </del>	6'-2"	
<del> </del>		• Vories	
	2" (42" PPB) -6" (54" PPB)	Φ = 8° for Bars 8T1 Φ = 5° for Bars 8T2	
Bars 8T2	4'-10''		<u> </u>
·	TRANSITION BAR	TRANSITION STIRRUP BARS 5P RS 8T1 & 8T2	Shoulde Paveme
	TRANSITION DAI	required per Railing End Transition)	Fill (see
10"		Contractor's 5	
33/4" 61/4"		option	
	61/4"	45°	
		/ / /     / /	
		54°30' +	
	, N		
		,	
		, 7	Barrie
			Gutte
	<b> </b>		Field i
	<del></del>		Bar 5
51/2"		Field Cut	mainto
<del>  • •  </del>		$1'-2\frac{1}{2}''$	
STIRRUP BAR 5	SP STIRRUP	STIRRUP BAR 5V END STIRRUP BAR 5V	Shoulde Paveme
SIINNUF DAR S	BAR 5R	To Be Field Cut	Fill (see
	2, 3, .	(Two required per	
		Barrier End Transition	
		w/ Tapered Toe)	









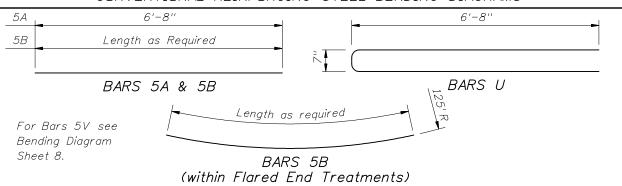
### 5 6'-8" Α As Read. В 5

U

14'-0''

3. The Contractor may use Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A 497.

### CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

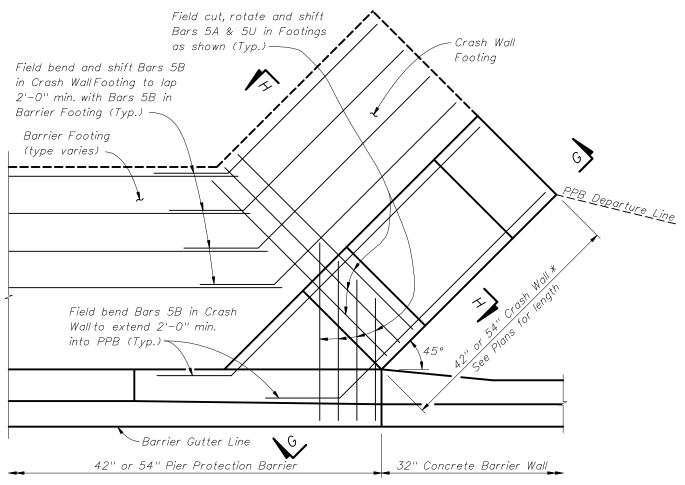


ESTIMATED BARRIER FOOTING QUANTITIES						
ITEM	UNIT	QUANTITY				
Concrete	CY/LF	0.335				
Reinforcing Steel (w/ Bars 5V) Cantilever Footing	LB/LF	64.32				
Reinforcing Steel (w/ Bars 5V) T Footing	LB/LF	63.01				

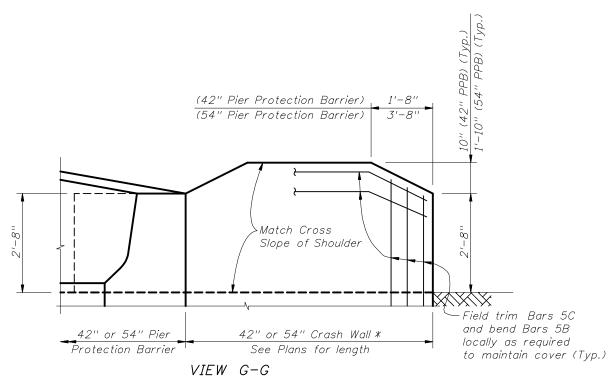
- 1. Provide 3" lip when optional construction joint is used. Omit 3" lip adjacent to Barrier Wall Inlets and as required to provide 2" min. clear between Cantilever Footing and adjacent Pier Footing or Column.
- 2. See Sheets 7 & 8 for Barrier Details.

### BARRIER FOOTING DETAILS

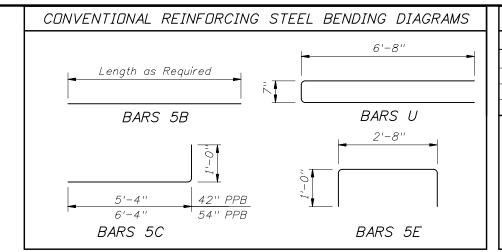
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SINTE OF FLORIDA	2010 FDOT Design Standards	Last Revision	Sheet No.
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TANK THE PARTY OF	PIER PROTECTION BARRIER	Index No.	
OF TRAIL		4	



PLAN VIEW (Concrete Barrier Wall Continuation shown, Guardrail Continuation similar)



\* Match height of adjacent Pier Protection Barrier



BILL OF REINFORCING STEEL						
MARK	SIZE	LENGTH				
В	5	As Reqd.				
С	5	6'-4" / 7'-4"				
Ε	5	4'-8"				
U	5	11'-0''				

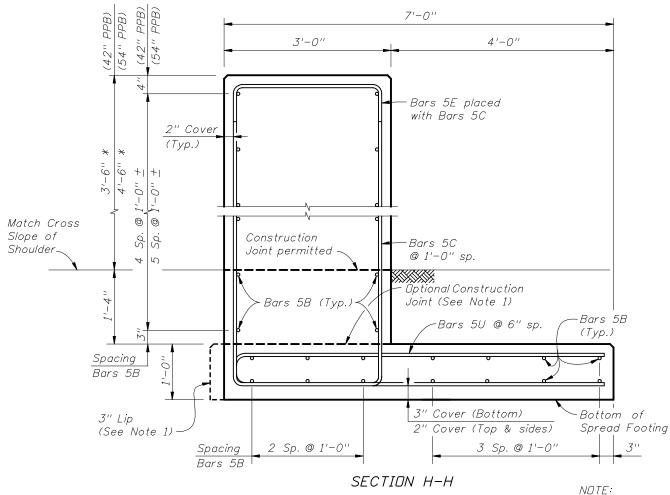
REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. Lap splices for Bars 5B shall be a minimum of 2'-2".
- 3. The Contractor may use Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A 497.

ESTIMATED CRASH WALL & FOOTING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete (Footing)	CY/LF	0.260	
Concrete (42" Crash Wall)	CY/LF	0.389	
Concrete (54" Crash Wall)	CY/LF	0.500	
Reinforcing Steel (42" Crash Wall)	LB/LF	66.06	
Reinforcing Steel (54" Crash Wall)	LB/LF	70.23	

### NOTES:

- 1. Provide 3" lip when optional construction joint is used.
- 2. See Sheet 8 for Barrier Details and Sheet 9 for Barrier Footing details.



# CRASH WALL & FOOTING DETAILS 2010 FDOT Design Standards

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PPB = Pier Protection Barrier



Index No.

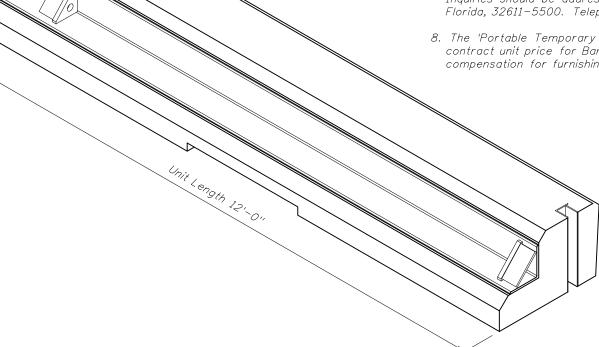
### GENERAL NOTES

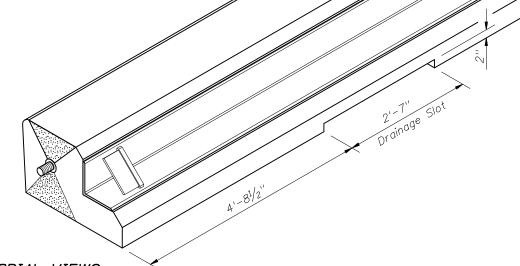
- 1. The 'Portable Temporary Low Profile Barrier For Roadside Safety' is a licensed design by the University Of Florida. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing (Index No. 412) is provided by the Florida Department Of Transportation solely for use by the Department and its assignees. The purpose for this standard drawing is to indicate the approval of use of the barrier on the State Highway System; to provide sufficient pictorials for identifying the barrier unit; and, to provide general installation geometry for the barrier.
- 3. Only those barrier units cast by producers licensed by the University Of Florida will be allowed for installation on the State Highway System in Florida.

Barrier wall units shall conform to Section 521 of the Standard Specification and shall be produced in Department approved plants with quality control plans for precasting concrete barrier walls. Each barrier wall unit shall be permanently marked with an identification that is traceable to the manufacturer, the producing precast concrete plant and the date of production. This permanent identification mark will serve as certification that the unit has been manufactured in accordance with University of Florida drawings and specifications, and the approved quality control program.

- 4. The low profile barrier is to be installed only with hardware and accessories furnished by the licensed barrier producer. Units shall be used for no purpose other than as interconnected segments in a run of barrier.

  Low profile barrier wall units shall be installed so as to be in firm contact with adjoining units. Nuts on tensioning rods shall be installed snug tight.
- 5. The low profile barrier is applicable for design speeds of 45 mph or less.
- 6. Tubular markers shall be installed along the run of barrier at the ends and at 50' centers on tangents and 25' centers on radii. The markers shall be fixed to the top of the barrier by an adhesive or other method approved by the engineer. Approach end units shall be marked with a Type I object marker. The cost of the tubular markers and Type I object marker shall be included in the cost of the low profile barrier.
- 7. Information regarding licensing, shop drawings, specifications, quality control and certification of compliance can be obtained from the University Of Florida, Office of Technology Licensing. Inquiries should be addressed to: Office of Technology Licensing, P.O. Box 115500, Gainesville, Florida, 32611-5500. Telephone: 352-392-8929, Fax: 352-392-6600. Reference UF#-11052.
- 8. The 'Portable Temporary Low Profile Barrier For Roadside Safety' shall be paid for under the contract unit price for Barrier Wall (Temporary) Low Profile Concrete, LF, and will be full compensation for furnishing, installing, maintaining and removing the barrier wall.



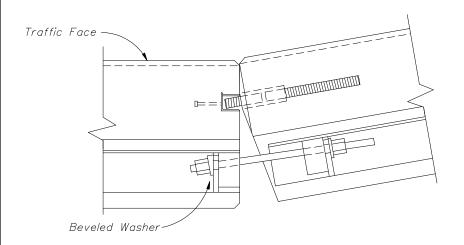


BACKSIDE AND END PICTORIAL VIEWS

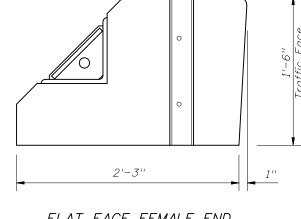
PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY



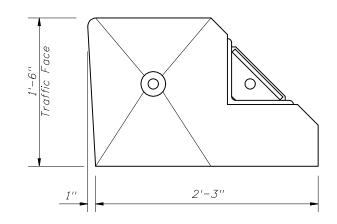
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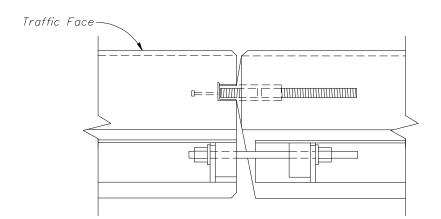
CONCAVE CONNECTION



FLAT FACE FEMALE END



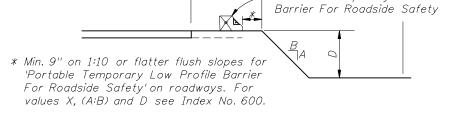
BEVELED FACE MALE END



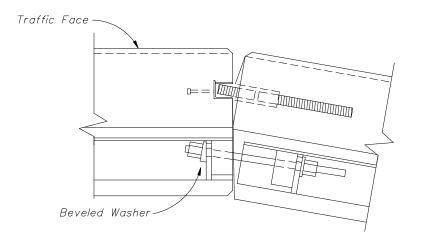
PARALLEL CONNECTION

## Clear Zone (CZ) Edge Of Travel Way --Portable Temporary Low Profile

END VIEWS

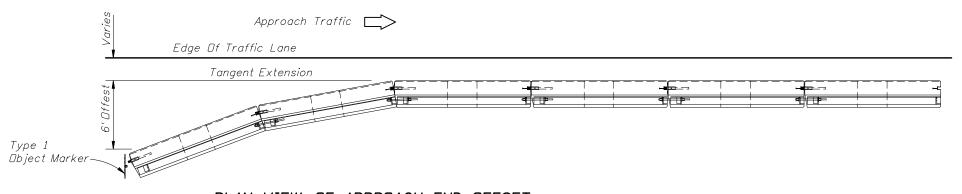


### DEFLECTION SPACE AT DROPOFFS



CONVEX CONNECTION

PLAN VIEWS OF CONNECTIONS



PLAN VIEW OF APPROACH END OFFSET

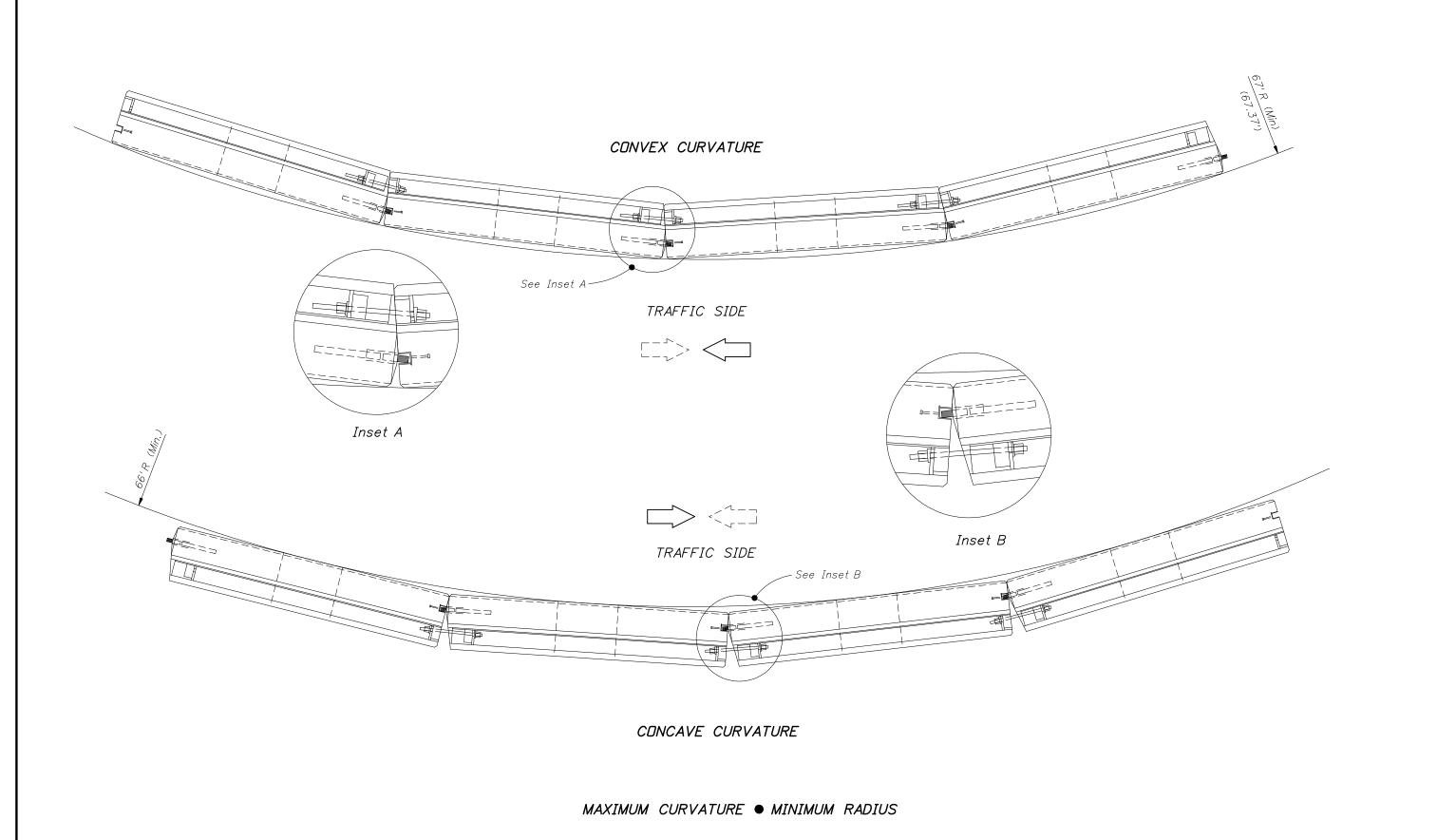
PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY



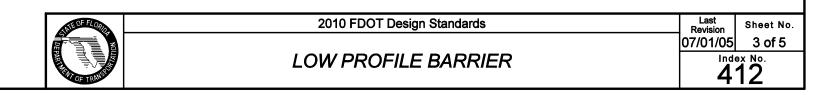
2010 FDOT Design Standards

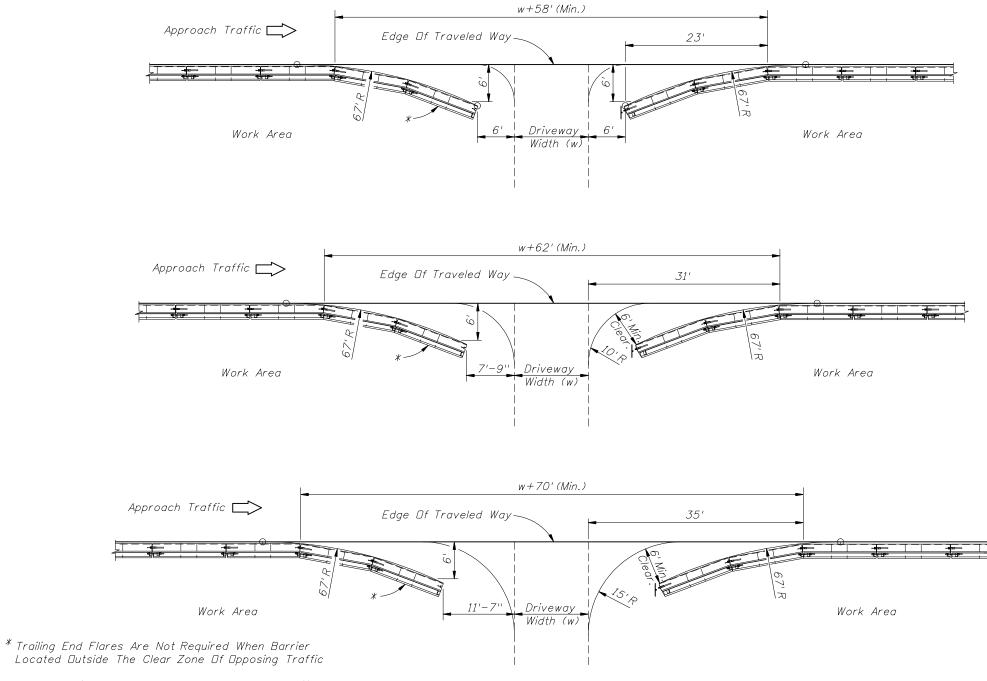
Sheet No. 07/01/05 2 of 5

LOW PROFILE BARRIER



PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY





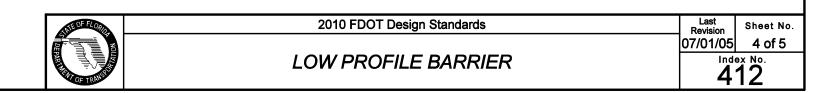
Type I Object Marker To Be Installed When Trailing End Flare Falls Within The Clear Zone Of Opposing Traffic

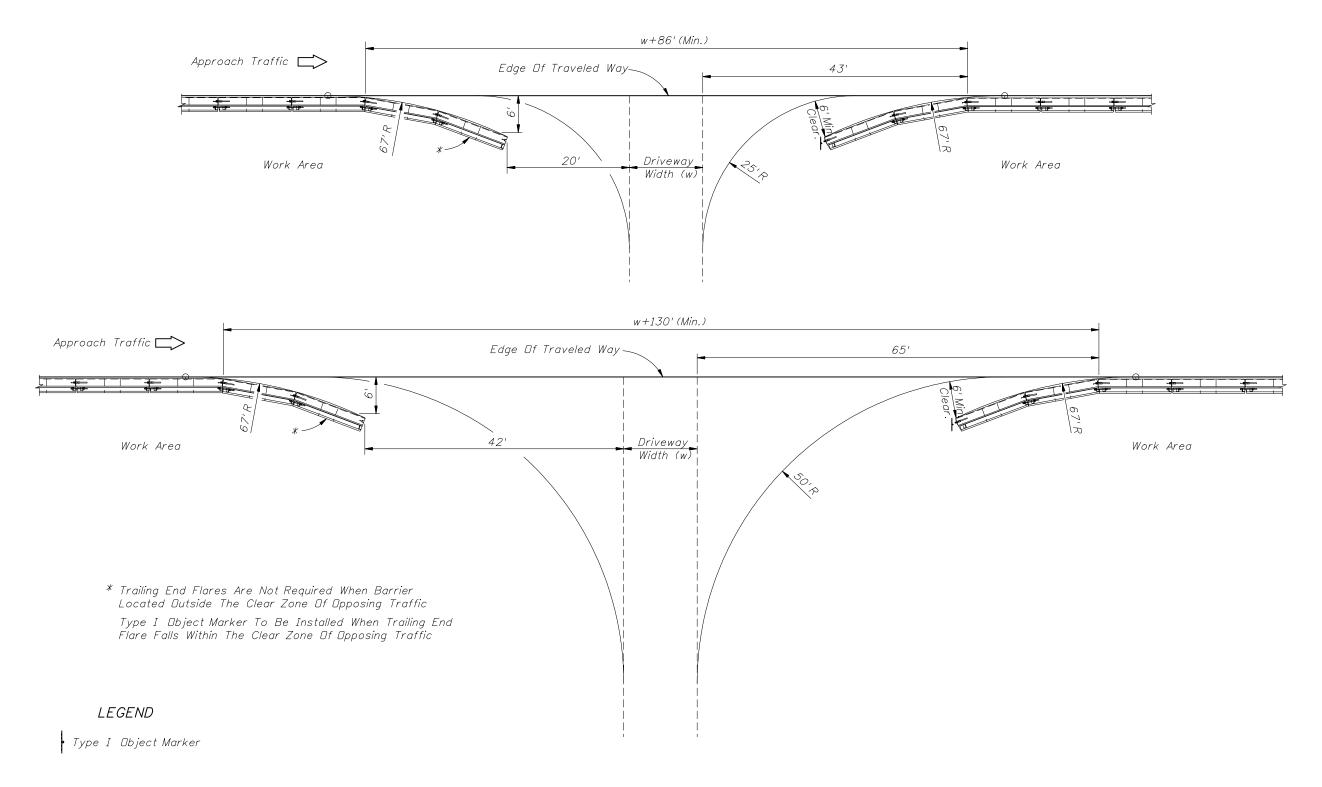
# LEGEND

BARRIER OPENINGS AT DRIVEWAYS

Type I Object Marker

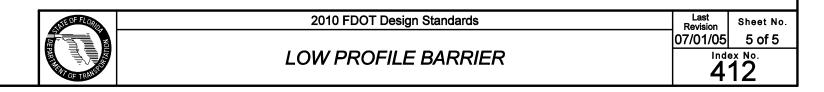
PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY





BARRIER OPENINGS AT DRIVEWAYS

# PORTABLE TEMPORARY LOW PROFILE BARRIER FOR ROADSIDE SAFETY



The Type K Temporary Concrete Barrier System has been crash tested to NCHRP Report 350 TL-3 criteria or structurally evaluated to meet the requirements of NCHRP Report 350 TL-3 criteria for the installation configurations as shown utilizing the types, sizes, lengths, shapes, strengths and grades of the fabrication and installation materials as shown.

In order to maintain crashworthiness of the system, do not substitute different grades, sizes, shapes or types of reinforcing steel for those shown for constructing Type K Barrier Units. Also, do not substitute different type, size, length or material grade anchor bolts, nuts, washers, adhesives, connector pins, stakes, keeper pins, or guardrail components for installing Type K Barrier Units.

#### FABRICATION NOTES:

- FABRICATOR PREQUALIFICATION: The Barrier Units shall be made in a prestressed concrete plant that meets the requirements of Specification Section 450 or in a precast plant meeting the requirements of Specification Section 6-8.
- CONCRETE: Concrete shall be Class IV in accordance with Specification Section 346. Specification Sections 346-10.2 through 346-10.4 are not applicable. Barrier Units represented by concrete acceptance strength tests which fall below 5000 psi will be rejected.
- REINFORCING STEEL: All reinforcing steel shall be ASTM A 615, Grade 60 except for Bars 6D1, 6D2 and 6D3. Bars 6D1, 6D2 and 6D3 shall be ASTM A 706 except that a  $2\frac{\pi}{4}$  diameter pin must be used for the 180 degree bend test. After fabrication, all or part of Bars 6D shall be hot dip galvanized in accordance with Specification Section 962 or coated with a cold galvanizing compound in accordance with Specification Section 975. The minimum limit of galvanizing or coating is shown in the Bending Diagrams. At the Fabricator's option, the entire length of Bars 6D may be galvanized or coated. Install Bars 6D within  $\frac{1}{8}$ " of the plan dimensions. Correct placement of Bars 6D is critical for proper fit up and performance of individual Barrier Units.

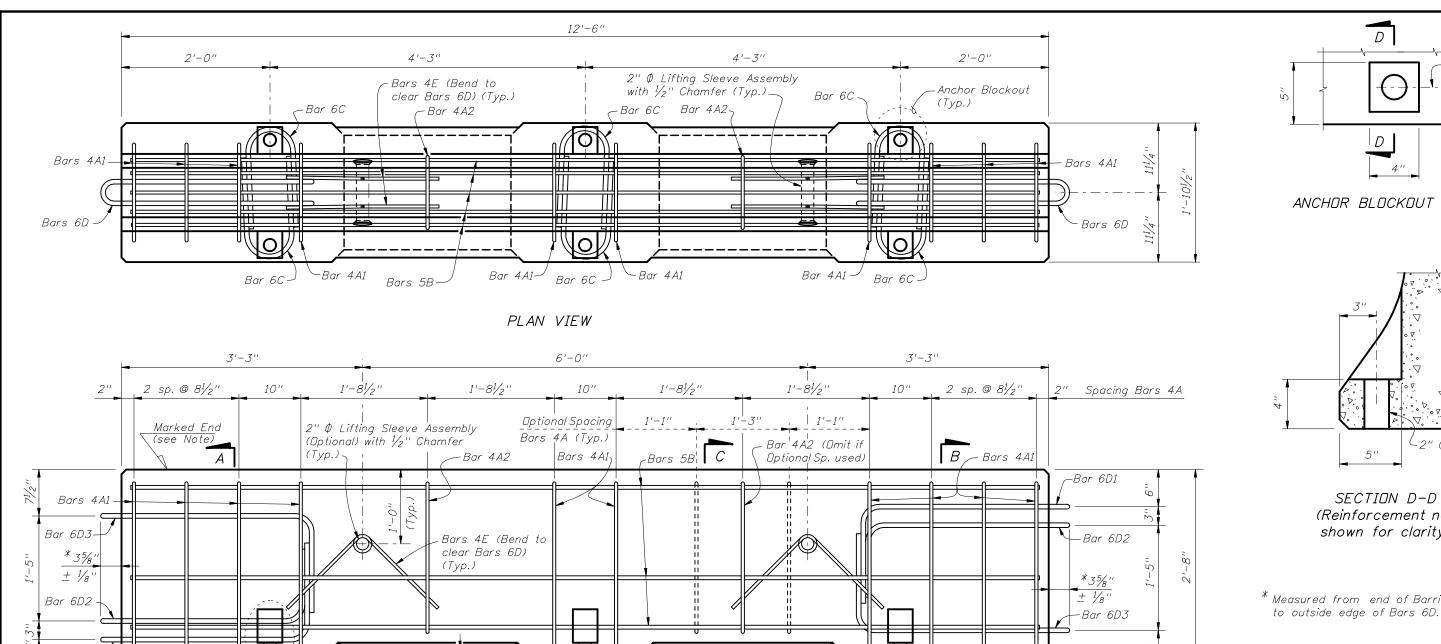
At the option of the Fabricator, Deformed Welded Wire Fabric in accordance with ASTM A 497 and the details shown on Sheet 2 may be utilized in lieu of Bars 4A and 5B.

All dimensions in the Bending Diagrams are out to out. All reinforcing steel shall have a 2" minimum cover except as noted.

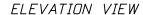
- LIFTING SLEEVE ASSEMBLY: Inclusion of the Lifting Sleeve Assemblies is optional. Steel for Pipe Sleeve shall be in accordance with ASTM A 53. Hot-dip galvanize the Lifting Sleeve Assemblies after their fabrication in accordance with the Specifications.
- SURFACE FINISH: Construct Barrier Units in accordance with Specification Sections 400 and 521. Finish the top and sides of the Barrier Units with a General Surface Finish. Finish the bottom of the Barrier Units to a dense uniform surface by floating in lieu of the General Surface Finish. Use stationary metal forms or stationary timber forms with a form liner.
- MARKING: Permanently mark the top left end of each Barrier Unit by the use of an embedded and anchored metallic plate with letters and figures a minimum of 0.5" tall. Ink stamps are not allowed. Permanently mark with the following information:
  - Type K1
  - Fabricator's name or symbol
  - Date of manufacture (day, month and year)
- HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.
- ALTERNATE DESIGN: Manufacturers seeking approval of proprietary concrete or steel barrier systems for inclusion on the Qualified Products List as pre-approved alternate designs must submit application along with design documentation showing the barrier system is crash tested to NCHRP Report 350 Test Level 3 criteria, is accepted by FHWA for use as a temporary concrete or steel barrier in the configurations shown herein, is a minimum of 2'-8" tall, has transitions and connections comparable to the standard design and has permanent deflections due to TL-3 crash test impacts not to exceed 3'-9" in freestanding configuration, 3.5" in bolted down configuration and 1'-0" in staked down configuration.



1ndex No. 414



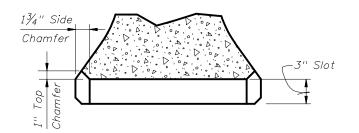
2'-3" (Lift/Drain Slot)



2'-0"

**% \** 

-Bars 6C



Anchor Blockout (both sides) (Typ.)

2'-3" (Lift/Drain Slot)

Bar 6D1 ~

SECTION THRU LIFT/DRAIN SLOT

ESTIMATED TEMPORARY CONCRETE BARRIER QUANTITIES					
ITEM UNIT QUANTITY					
Concrete	CY	1.29			
Reinforcing Steel	LB	218			

The above quantities are for one Barrier Unit.

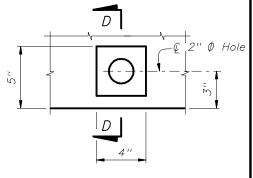
Cross References: For Section A-A, Section B-B and Section C-C see Sheet 3.

 $\mathcal{C}_{\mathbf{1}}$ 

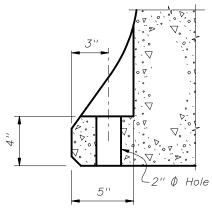
Bars 6C

В

3'-0"

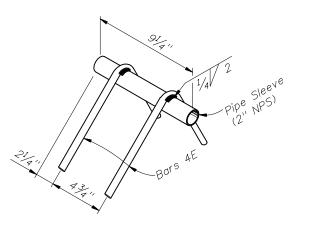


ANCHOR BLOCKOUT DETAIL



(Reinforcement not shown for clarity)

\* Measured from end of Barrier Unit



LIFTING SLEEVE ASSEMBLY DETAIL (OPTIONAL)



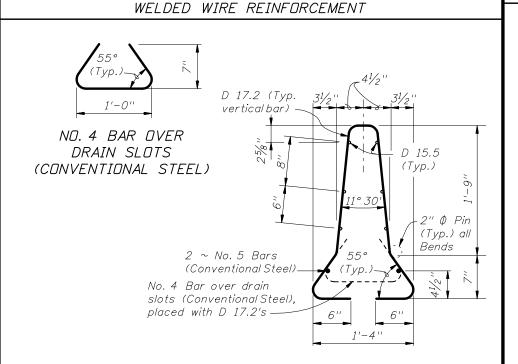
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TYPE K TEMPORARY CONCRETE BARRIER SYSTEM

Index No. 414

Sheet No.



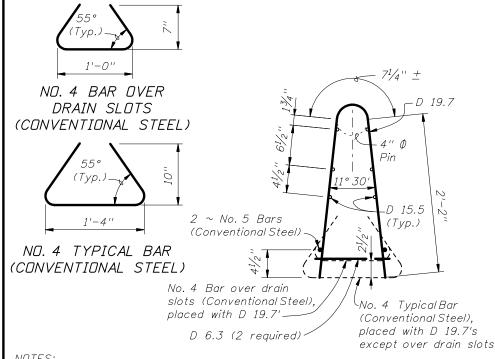
ALTERNATE REINFORCING STEEL DETAIL

#### NOTES:

Place 2 ~ No. 5 Bars (12'-3" long) in bottom of Welded Wire Reinforcement cage as shown.

D 17.2 spacing shall match spacings for Bars 4A shown in Elevation View, Sheet 2. Field trim D 17.2's to clear drain slots by 2".

#### CONFIGURATION ONE



NOTES:

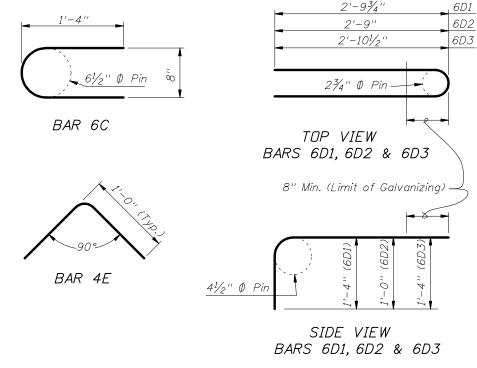
Place 2  $\sim$  No. 5 Bars (12'-3" long) in bottom of Welded Wire Reinforcement cage as shown.

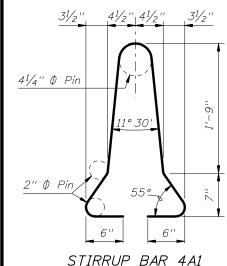
D 19.7 spacing shall match spacings for Bars 4A shown in Elevation View, Sheet 2. Field trim D 19.7's to clear drain slots by 2".

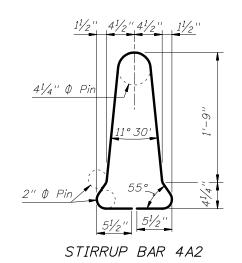
#### CONFIGURATION TWO

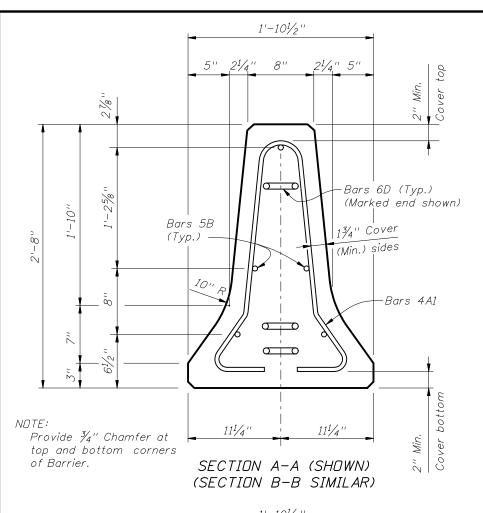
# CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS OF DEINEODOING CITE

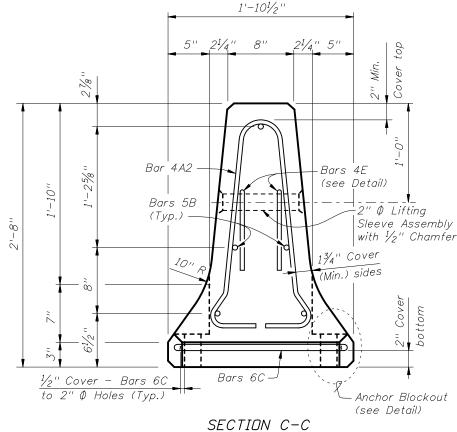
BILL	OF REINFORCING STEEL					
MARK	SIZE	NUMBER	LENGTH			
A1	4	10	6'-1''			
A2	4	2	5'-5''			
В	5	5	12'-3" (Straight)			
С	6	6	3'-1''			
D1	6	2	8'-4''			
D2	6	2	7'-6''			
D3	6	2	8'-6"			
Ε	4	4	2'-0''			









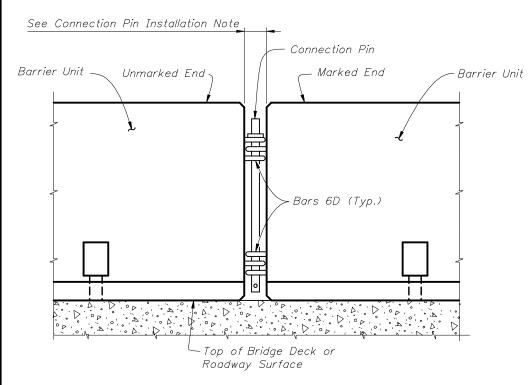


(Bars 6D not shown for clarity)

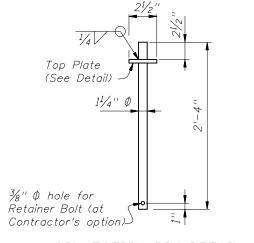


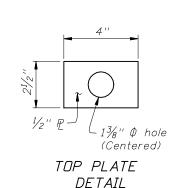
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DETAIL OF CONNECTION BETWEEN BARRIER UNITS





CONNECTION PIN DETAIL

NOTES FOR ALL INSTALLATIONS:

LIMITATION OF USE: This Temporary Concrete Barrier System is intended for work zone traffic control and other temporary applications. It shall not be used for permanent traffic railing construction unless specifically permitted by the Plans. Except as shown for the Back Filled Roadway Installations, the Barrier Units must be installed on a flexible pavement (asphalt) or rigid pavement (concrete) surface as shown with a cross slope of 1:10 or flatter. Except as shown for transition installations, Type K Barrier Units are not intended to be bolted down or staked down in locations where they can be impacted from the back side.

HANDLING: At no time shall the Barrier Units be lifted or moved by use of Bars 6D that extend from the ends of the units. Approximate weight of one unit equals 2.7 tons.

- SURFACE PREPARATION: Except as shown for the Back Filled Roadway Installations, remove all debris, loose dirt and sand from the pavement, bridge deck or Asphalt Pad surface within the barrier footprint just prior to placement of the Barrier Units.
- CONNECTION PIN ASSEMBLY: Steel for Connection Pin and Top Plate assemblies shall be in accordance with ASTM A36 or ASTM A709 Grade 36. Nondestructive testing of welds shall not be required. At the Contractor's option, a 3/8" diameter hole may be provided at the bottom of the Connection Pin, as shown, for the installation of a vandal resistance bolt.
- CONNECTION PIN INSTALLATION: Initially set Barrier Units by using a 35%" wooden block between ends of adjacent units. Install Connection Pin between adjacent Barrier Units as shown, then pull newly placed Barrier Unit away from adjacent Barrier Unit to remove slack between Connection Pin and Bars 6D (except as shown on Sheet 5). Barrier Units shall not be used unconnected.
- DELINEATION: Mount Type C Steady-Burn Lights on top of Barrier Units that are used as traffic barriers along travel ways in work zones. Space the lights at 50' centers in transitions, 100' centers on curves and 200' centers on tangent alignments. Refer to "Warning Lights" on Index No. 600 for additional information.
- REUSE OF UNITS: Barrier Units may be reused provided they have the structural integrity and surface qualities of new units. Do not use Barrier Units without Marking Plates.
- REUSE OF CONNECTION PINS: Connection pins may be reused if they have the structural integrity of new pins.
- INSTALLATIONS ON CURVED ALIGNMENTS: The details presented in these Standards are shown for installations on tangent alignments.

  Details for horizontally curved alignments are similar.
- TRANSITIONS: Transitions are required between freestanding, bolted down, staked down and back filled Type K Barrier installations, see
  Sheet 8 for transition requirements and details. Transitions are also required between installations of Type K Barrier and other types of
  temporary barrier, see Index No. 415 for transition requirements and details. Splices and transitions are required between installations
  of Type K Barrier and permanent Bridge or Roadway Traffic Railings, see Sheets 9 through 13 for transition requirements and details.
- PAYMENT: Barrier Units for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary) (F&I) (Type K), LF. Any relocation of the Barrier Units required for the project shall be paid for under the contract unit price for Barrier Wall (Temporary) (Relocate) (Type K), LF. Type C Steady—Burn Lights shall be paid for under the contract unit price for Lights (Temp. Barrier Wall Mount) (Type C, Steady Burn), ED. The Contractor shall furnish Barrier Units except when the Plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return. Unless otherwise noted on the Plans, the Barrier Units shall become the property of the Contractor and shall be removed from the site prior to acceptance of the completed project.

NOTES FOR THRIE BEAM GUARDRAIL SPLICE INSTALLATIONS:

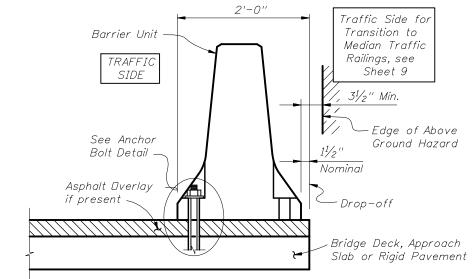
THRIE-BEAM GUARDRAIL: Provide Thrie-Beam Guardrail for splices in accordance with AASHTO M 180, Type II (Zinc coated) and as follows:
Two panels per splice (One panel per side) of Class B (10 Gauge), or
Four panels per splice (Two nested panels per side) of Class A (12 Gauge).
Guardrail panel length shall be 12'-6". Provide and install all other associated metallic guardrail components (Terminal

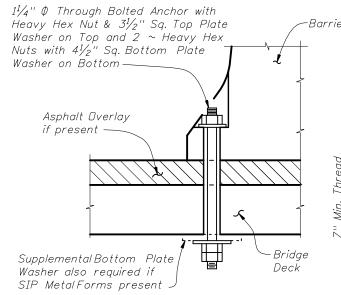
Connectors, Shoulder Bolts, Hex Bolts and Nuts, Filler Plates, etc.) in accordance with Index No. 400.

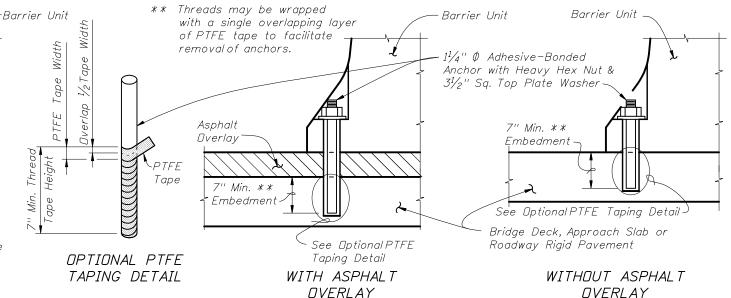
Install five Guardrail Anchor Bolts at each end of each splice in any of the standard seven anchor bolt holes in the Thrie-Beam Terminal Connector. If reinforcing steel is encountered when drilling holes for Guardrail Anchor Bolts in Type K Barrier Units, shift Thrie-Beam Terminal Connector so as to clear reinforcing steel within the given tolerances or select a different bolt hole to use. Do not drill or cut through reinforcing steel within Type K Barrier Units. Drilling or cutting through reinforcing steel within permanent concrete traffic railings is permitted. Do not drill or cut through utilities or conduits within permanent concrete traffic railings.

- GUARDRAIL OFFSET BLOCKS: Provide and install timber Offset Blocks meeting the material requirements of Index No. 400. Field trim Offset Blocks as required for proper fit. Utilize Offset Blocks as shown and required in order to prevent bending or kinking of Thrie-Beam Guardrail panels.
- CDNCRETE FOR FILLING TAPERED TRAFFIC RAILING TOES: Provide concrete for filling tapered toes of Traffic Railings as shown meeting the material requirements of Specification Section 346, any Class, or a commercially available prebagged concrete mix (3000 psi minimum compressive strength). Sampling, testing, evaluation and certification of the concrete in accordance with Specification Section 346 is not required. Saturate with water the surfaces upon and against which the concrete fill will be placed prior to placing concrete. Place and finish concrete fill using forms or by hand methods to the general configurations shown so as to provide a smooth shape transition between the Type K Barrier and the adjacent traffic railing. A low slump is desirable if placing and finishing concrete by hand methods. Cure the concrete fill by application of a curing compound, or by covering with a wet tarp or burlap for a minimum of 24 hours. Completely remove the concrete fill upon relocation or removal of the Type K Temporary Concrete Barrier.









TYPICAL SECTION (BRIDGE DECK SHOWN, APPROACH SLAB OR RIGID PAVEMENT SIMILAR; INSTALLATION ADJACENT TO DROP-OFF SHOWN, MEDIAN TRANSITION INSTALLATION SIMILAR)

THROUGH BOLTED ANCHOR INSTALLATION ON BRIDGE DECK

NOTES FOR BOLTED DOWN BRIDGE, APPROACH SLAB, ROADWAY AND TRANSITION INSTALLATIONS:

LIMITATION OF USE: This installation technique can only be used on rigid pavement and concrete bridge decks as shown.

Barrier Units shall not be bolted down on bridge superstructures that contain post-tensioned tendons within the concrete deck (top flange of concrete box girders) or on bridge superstructures consisting of longitudinally prestressed, transversely post-tensioned, solid or voided concrete slab units. Anchor Bolts must not be installed on both sides of the Barrier Units. Do not bolt down Barrier Units across bridge finger or modular expansion joints.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive—Bonded Anchor Bolts shall be fully threaded rods in accordance with ASTM F 1554 Grade 36. Anchor Bolts for through bolting shall be in accordance with ASTM A 307 or ASTM F 1554 Grade 36. Nuts shall be in accordance with ASTM A 563 or ASTM A 194. Flat Washers shall be in accordance with ASTM F 436 and Plate Washers shall be in accordance with ASTM A 36 or ASTM A 709 Grade 36.

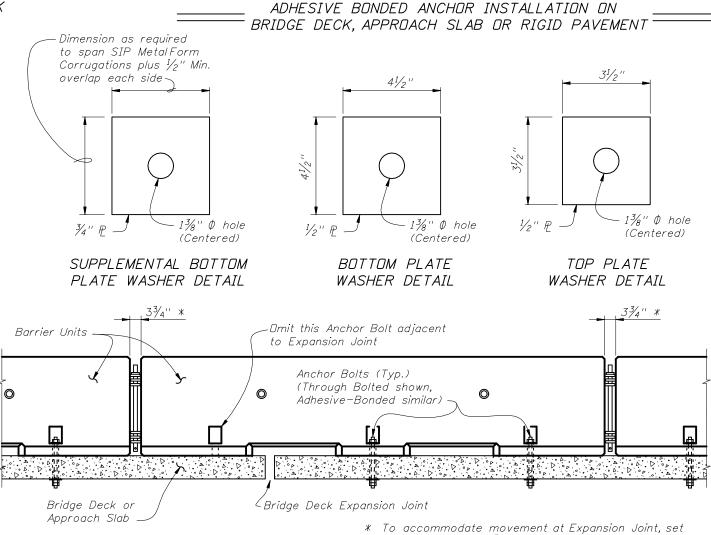
Install three (3) Anchor Bolts per Barrier Unit on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of Anchor Bolts required in Transition Installations see Sheets 8 and 9 and Index No. 415. Drilling through deck reinforcing steel to install Anchor Bolts is permitted. Unless otherwise shown in the Plans, at the Contractor's option Barrier Units may be installed by through bolting (where geometrically possible) or by the use of Adhesive-Bonded Anchor Bolts. Do not drill into or otherwise damage the tops of supporting beams or girders, bridge deck expansion joints or drains. Install Anchor Bolts and Nuts so that the maximum extension beyond the face of the Barrier Units is  $\frac{1}{2}$ ". Snug tighten the Nuts on the Anchor Bolts. For through bolted installations, snug tighten the double Nuts on the underside of the deck against each other to minimize the potential for loosening.

Omit one (1) Anchor Bolt within a single Barrier Unit if a conflict exists between the Anchor Bolt location and a bridge deck expansion joint or drain. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

Omit one (1) Anchor Bolt within a single Barrier Unit as shown in the Treatment at Bridge Deck Expansion Joint Schematic if the Barrier Unit straddles a bridge deck expansion joint. The adjacent Barrier Units must each be installed with the standard three (3) Anchor Bolts.

ADHESIVE-BONDING MATERIAL SYSTEMS: Adhesive Bonding Material Systems for Anchor Bolts shall be Type HSHV in accordance with Specification Section 937 and shall be installed in accordance with Specification Section 416. Prior to installation of the Barrier Units in the Plan location(s), install a demonstration Barrier Unit using the proposed production installation method, at a location approved by the Engineer. In lieu of the production test requirements of Specification Section 416-6, install six (6) Adhesive-Bonded Anchor Bolts in the demonstration Barrier Unit and test each Anchor Bolt with a 29,800 pound tensile proof load. Install and test additional demonstration Barrier Units when requested by the Engineer. Remove the demonstration Barrier Unit prior to testing the Anchor Bolts. Remove the test Anchor Bolts after testing as directed by the Engineer.

REMOVAL OF ANCHOR BOLTS: Upon removal or relocation of Barrier Units, remove all Anchor Bolts and completely fill the remaining holes in bridge decks, approach slabs and roadway rigid pavements that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.

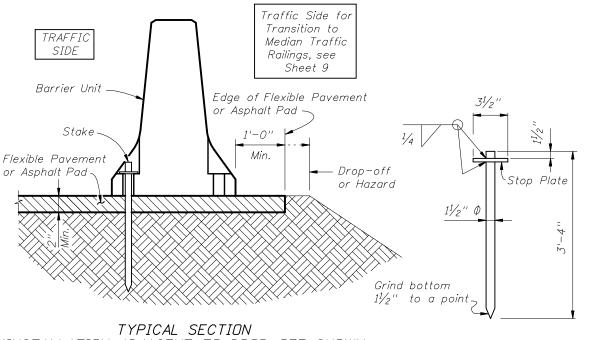


TREATMENT AT BRIDGE DECK EXPANSION JOINT SCHEMATIC

Barrier Units with  $3\frac{3}{4}$ " gap at locations shown.

BOLTED DOWN BRIDGE, APPROACH SLAB, ROADWAY AND TRANSITION INSTALLATIONS





31/2" 15/8" ∅ hole (Centered)

(INSTALLATION ADJACENT TO DROP-OFF SHOWN, STAKE DETAIL MEDIAN TRANSITION INSTALLATION SIMILAR)

STOP PLATE DETAIL

NOTES FOR STAKED DOWN ROADWAY AND TRANSITION INSTALLATIONS:

- LIMITATION OF USE: This installation technique can only be used on flexible pavement or an Asphalt Pad as shown. Stakes must not be installed on both sides of the Barrier Units.
- ASPHALT PAD: Where existing flexible pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.
- STAKES: Provide steel for Stake assemblies in accordance with ASTM A 36 or ASTM A 709 Grade 36. All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal shall be E60XX or E70XX. Nondestructive testing of welds is not required.

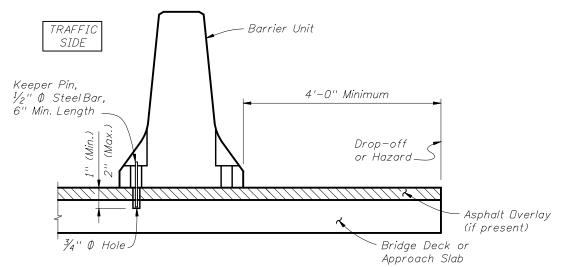
Install three (3) Stakes on the traffic side of the Barrier Units as shown, except for Transition Installations. For the number and positions of stakes required in Transition Installations see Sheets 8 and 9 and Index No. 415. Install Stakes so that the Stop Plate is snug against the bottom of the Anchor Blockout.

BURIED UTILITIES: Prior to installation of Stakes verify locations of all adjacent buried utilities, drainage structures, pipes, etc. If conflicts between Stake locations and buried elements exist, a maximum of two (2) Stakes within a single Barrier Unit may be omitted if the adjacent Barrier Units are installed with the standard three (3) Stakes.

REMOVAL OF STAKES: Upon removal or relocation of Barrier Units, completely remove all Stakes and completely fill the remaining holes in flexible pavement that is to remain with hot or cold patch asphalt material.

REUSE OF STAKES: Stakes may be reused if they have the structuralintegrity of new stakes.

# = STAKED DOWN ROADWAY AND TRANSITION INSTALLATIONS 💳



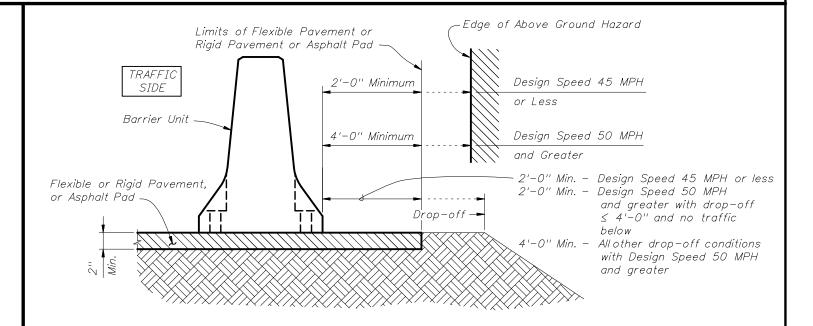
TYPICAL SECTION (BRIDGE DECK SHOWN, APPROACH SLAB SIMILAR)

NOTES FOR FREE STANDING BRIDGE OR APPROACH SLAB INSTALLATIONS:

KEEPER PINS: Keeper Pins shall be  $\frac{1}{2}$ " diameter, smooth steel bar in accordance with ASTM A 36 or ASTM A 709 Grade 36. As directed by the Engineer in order to limit vibration induced translation of the Barrier Units, install one (1) Keeper Pin per Barrier Unit on the traffic side of the Barrier Units as shown. Do not drill into or otherwise damage bridge deck expansion joints or drains.

REMOVAL OF KEEPER PINS: Upon removal or relocation of Barrier Units, remove all Keeper Pins and completely fill the remaining holes in bridge decks and approach slabs that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.

FREESTANDING BRIDGE OR APPROACH SLAB INSTALLATIONS =



# TYPICAL SECTION

NOTES FOR FREE STANDING ROADWAY INSTALLATION:

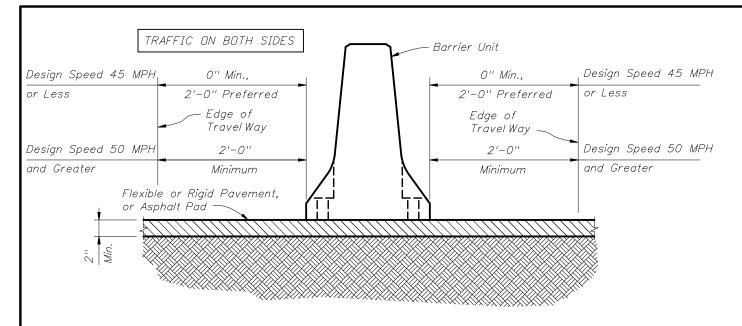
LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement or on an Asphalt Pad as shown.

ASPHALT PAD: Where existing pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

# FREESTANDING ROADWAY INSTALLATION =



414



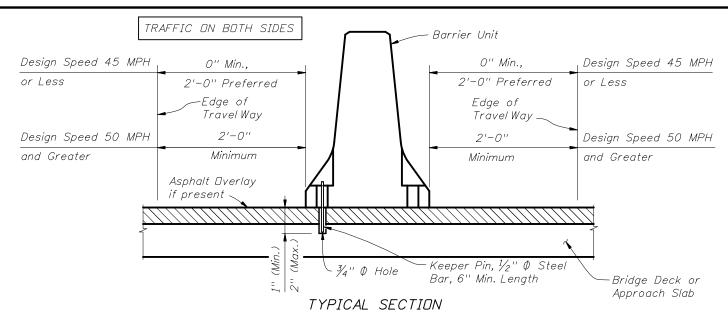
#### TYPICAL SECTION

NOTES FOR FREE STANDING ROADWAY MEDIAN INSTALLATION:

LIMITATION OF USE: This installation technique can only be used on flexible or rigid pavement or on an Asphalt Pad as shown.

ASPHALT PAD: Where existing pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement in accordance with Specification Section 339 with the exception that the use of a pre-emergent herbicide is not required. No separate payment will be made for the Asphalt Pad.

# = FREESTANDING ROADWAY MEDIAN INSTALLATION =



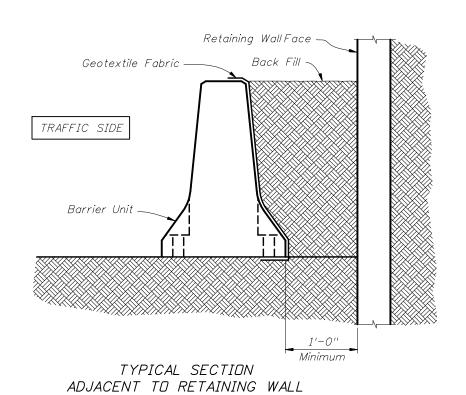
NOTES FOR FREE STANDING BRIDGE OR APPROACH SLAB MEDIAN INSTALLATION:

KEEPER PINS: Keeper Pins shall be  $\frac{1}{2}$ " diameter, smooth steel bar in accordance with ASTM A 36 or ASTM A 709 Grade 36.

As directed by the Engineer in order to limit vibration induced translation of the Barrier Units, install one (1) Keeper Pin per Barrier Unit as shown. Alternate Keeper Pin locations from side to side of Barrier Units along the length of the installation. Do not drill into or otherwise damage bridge deck expansion joints or drains.

REMOVAL OF KEEPER PINS: Upon removalor relocation of Barrier Units, remove all Keeper Pins and completely fill the remaining holes in bridge decks and approach slabs that are to remain with Magnesium Ammonium Phosphate Concrete in accordance with Specification Section 930 or with an Epoxy Resin Compound, Type I or Q, in accordance with Specification Section 926. If a flexible pavement overlay is present and is to remain, completely fill the remaining holes in the flexible pavement with hot or cold patch asphalt material.

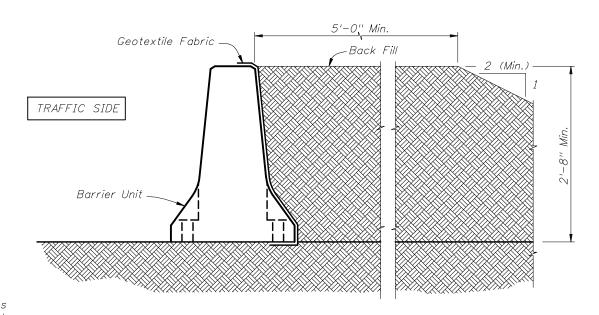
# =FREESTANDING BRIDGE OR APPROACH SLAB MEDIAN INSTALLATION=



NOTES FOR BACK FILLED ROADWAY INSTALLATIONS

BACK FILL MATERIAL: Provide Back Fill Material consisting of any available clean soil. Compact Back Fill Material until the soil mass is firm and unyielding. Provide erosion control as specified in the Plans. If none is specified in the Plans, provide erosion control as required to maintain the integrity of the Back Fill embankment.

GEDTEXTILE FABRIC: Provide Type D-5 Geotextile Fabric in accordance with Index No. 199 to contain Back Fill Material behind Barrier Units. Geotextile Fabric may be continuous over the length and height of the installation or may be individual pieces as required to cover the Lift / Drain Slots and open vertical joints between Barrier Units.



TYPICAL SECTION

:BACK FILLED ROADWAY INSTALLATIONS=

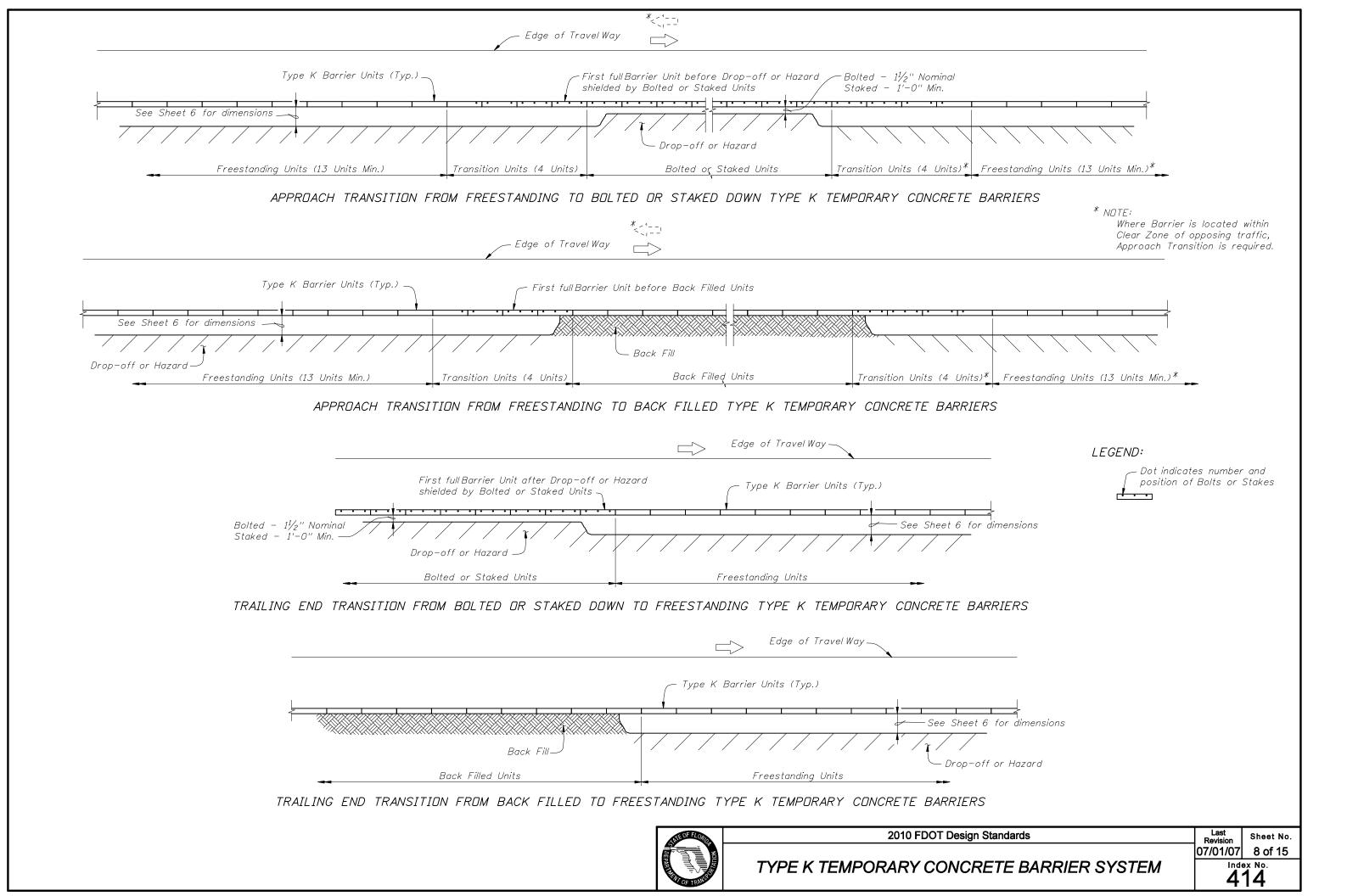
STE OF FLORIDA

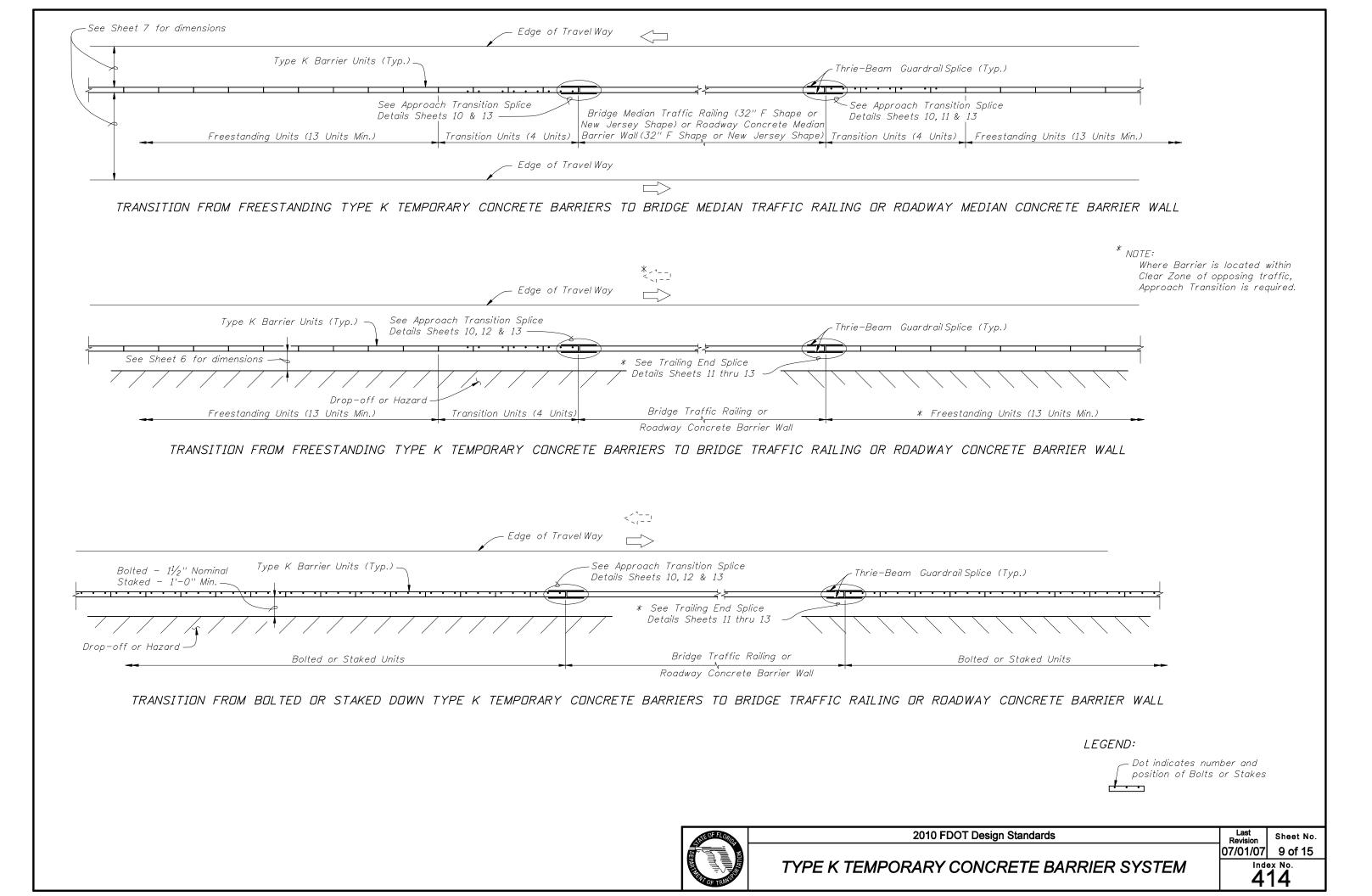
#### 2010 FDOT Design Standards

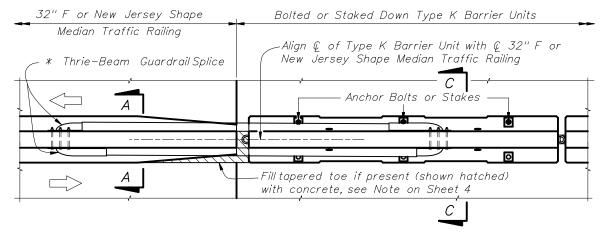
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TYPE K TEMPORARY CONCRETE BARRIER SYSTEM







PARTIAL PLAN VIEW AT MEDIAN TRAFFIC RAILING

See Sheet 13 for Section A-A,
New Jersey Shape and 42" F Shape Traffic
Railings and 8' or 14' Traffic Railing / Sound
Barriers (similar)

\* Thrie-Beam Guardrail Splice

\* Thrie-Beam Guardrail Splice

A Diffset Block bolted B

C Diffset Block

Anchor Bolts or Stakes

Align Top of Type K Barrier Unit with Traffic Railing at its end

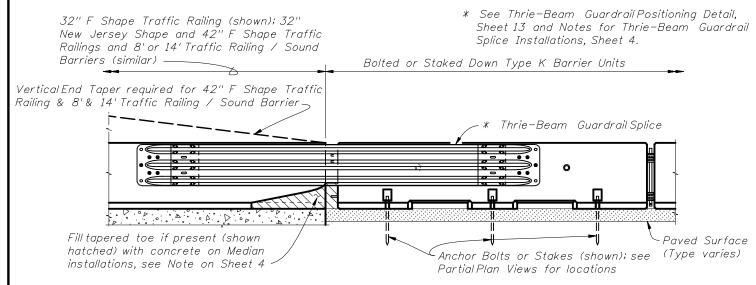
B

C Diffset Block

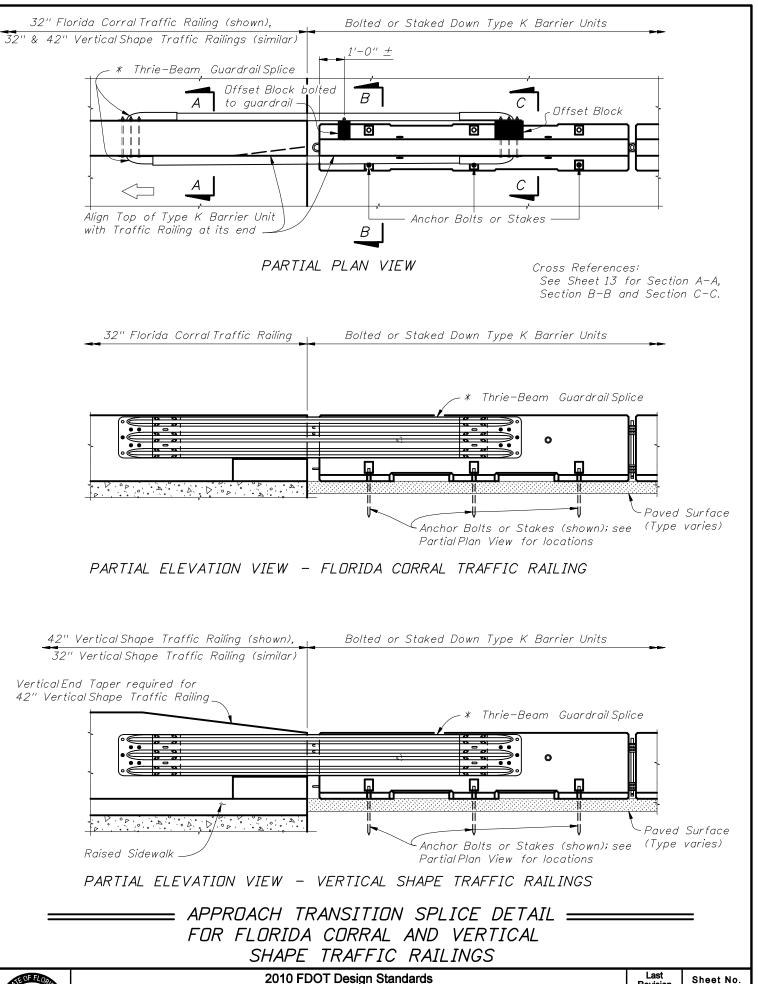
Align Top of Type K Barrier Unit with Traffic Railing at its end

Cross References:

PARTIAL PLAN VIEW AT SHOULDER TRAFFIC RAILING



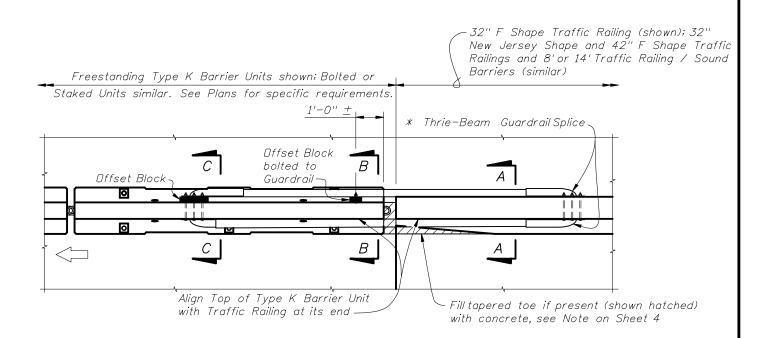
PARTIAL ELEVATION VIEW



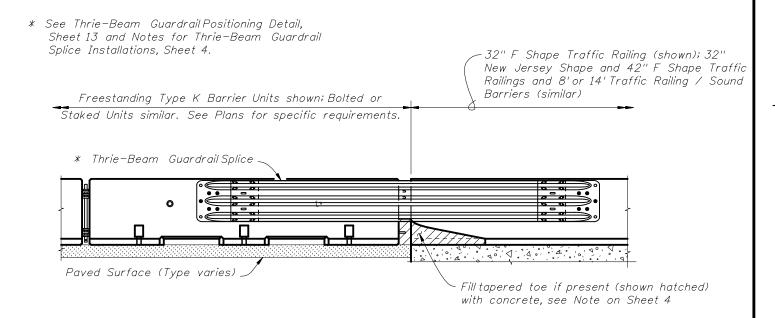


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#### PARTIAL PLAN VIEW



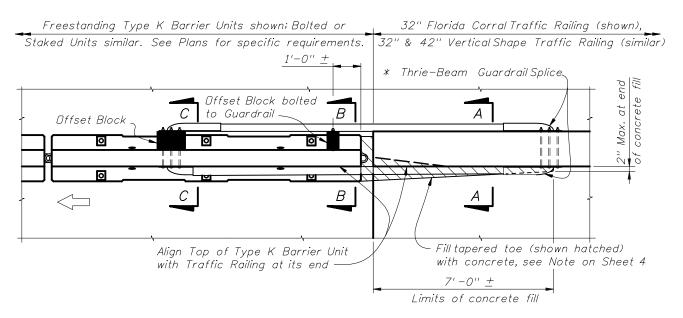
#### PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section A-A, Section B-B and Section C-C.

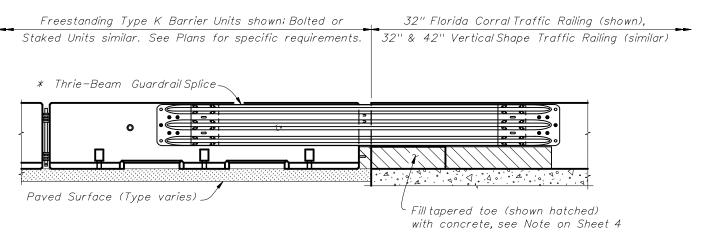
TRAILING END SPLICE DETAIL

FOR F AND NEW JERSEY SHAPE TRAFFIC RAILINGS

AND 8'& 14' TRAFFIC RAILING / SOUND BARRIERS



PARTIAL PLAN VIEW



#### PARTIAL ELEVATION VIEW

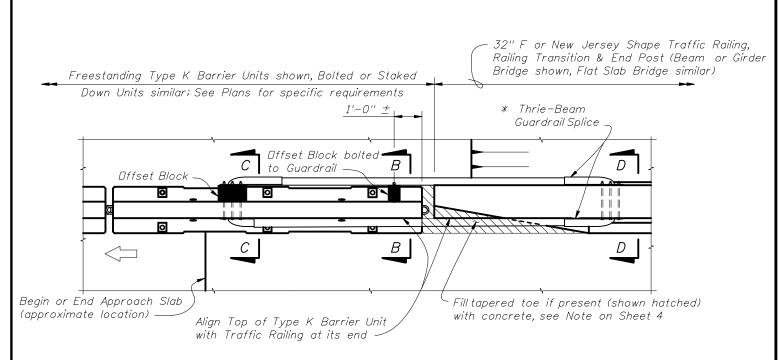
Cross References: See Sheet 13 for Section A-A, Section B-B and Section C-C.

=== TRAILING END SPLICE DETAIL ==== FOR FLORIDA CORRAL AND VERTICAL SHAPE TRAFFIC RAILINGS



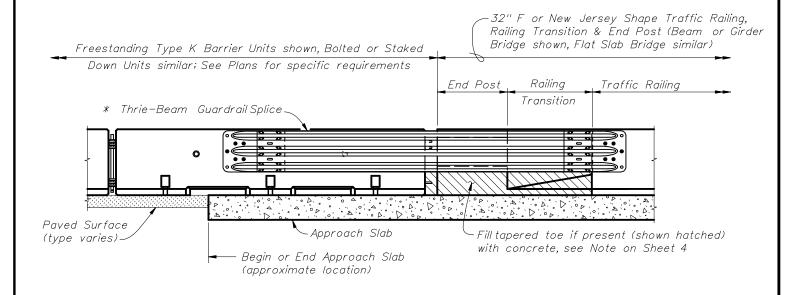
# 2010 FDOT Design Standards

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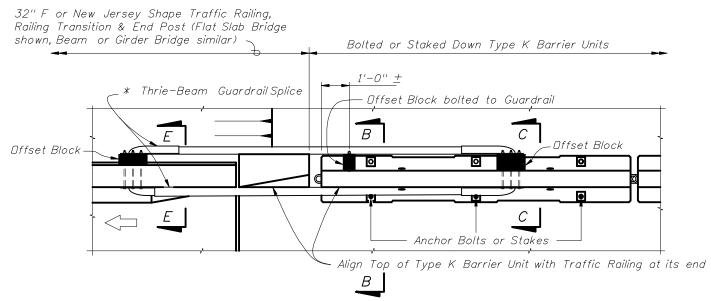
#### PARTIAL PLAN VIEW

\* See Thrie-Beam Guardrail Positioning Detail, Sheet 13 and Notes for Thrie-Beam Guardrail Splice Installations, Sheet 4.

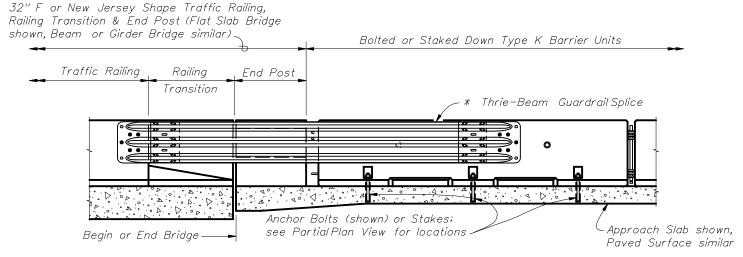


#### PARTIAL ELEVATION VIEW

Cross References: See Sheet 13 for Section B-B, Section C-C and Section D-D.



#### PARTIAL PLAN VIEW



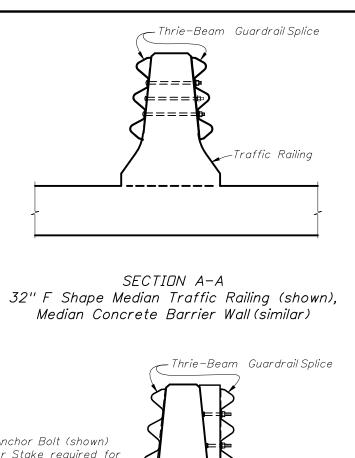
#### PARTIAL ELEVATION VIEW

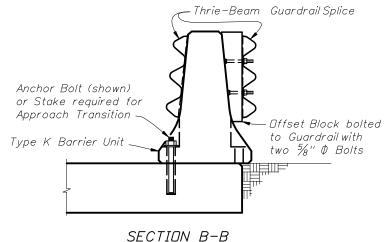
Cross References: See Sheet 13 for Section B-B, Section C-C and Section E-E.

————— APPROACH TRANSITION SPLICE DETAIL ———— FOR 32" F AND NEW JERSEY SHAPE TRAFFIC RAILINGS WITH RAILING TRANSITION AND END POST



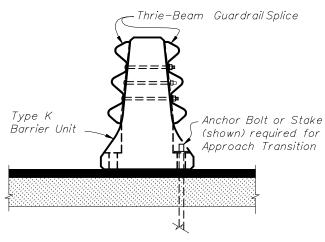
#### 2010 FDOT Design Standards



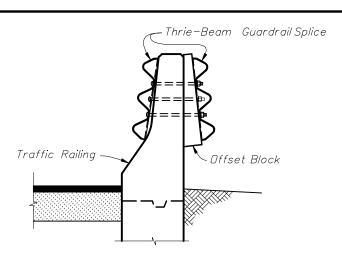


Adjacent to Shoulder Traffic Railings

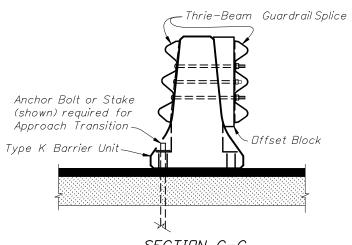
SECTION A-A 32" F Shape Traffic Railing (shown), 42" Traffic Railing and 8'& 14' Traffic Railing / Sound Barriers (similar)



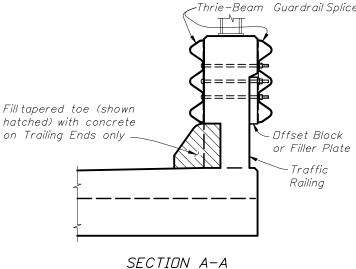
SECTION C-C Adjacent to 32" F or New Jersey Shape Median Traffic Railing or Median Concrete Barrier Wall



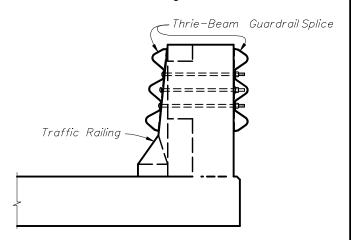
SECTION A-A
32" New Jersey Shape Concrete Barrier
Wall (shown), 32" New Jersey Shape Traffic
Railing & other Narrow Traffic Railings (similar)



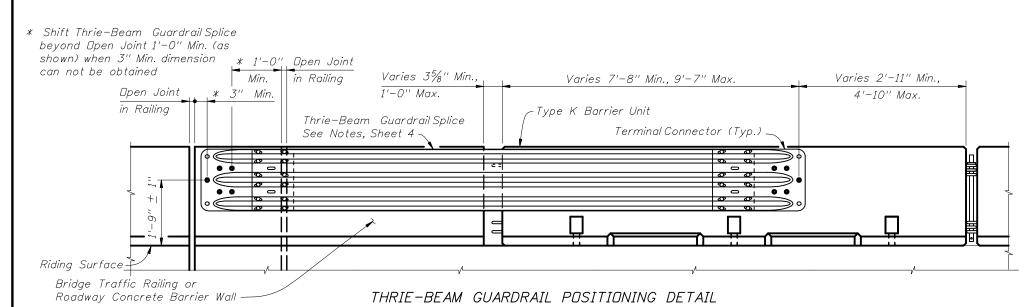
SECTION C-C Adjacent to Shoulder Traffic Railings

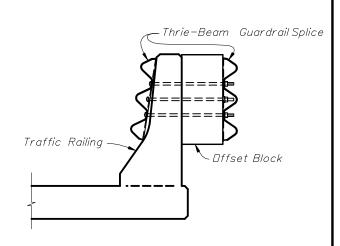


SECTION A-A 32" & 42" Vertical Shape Traffic Railing (shown), Florida Corral Traffic Railing (similar)



SECTION D-D 32" F or New Jersey Shape Traffic Railing, Railing Transition & End Post





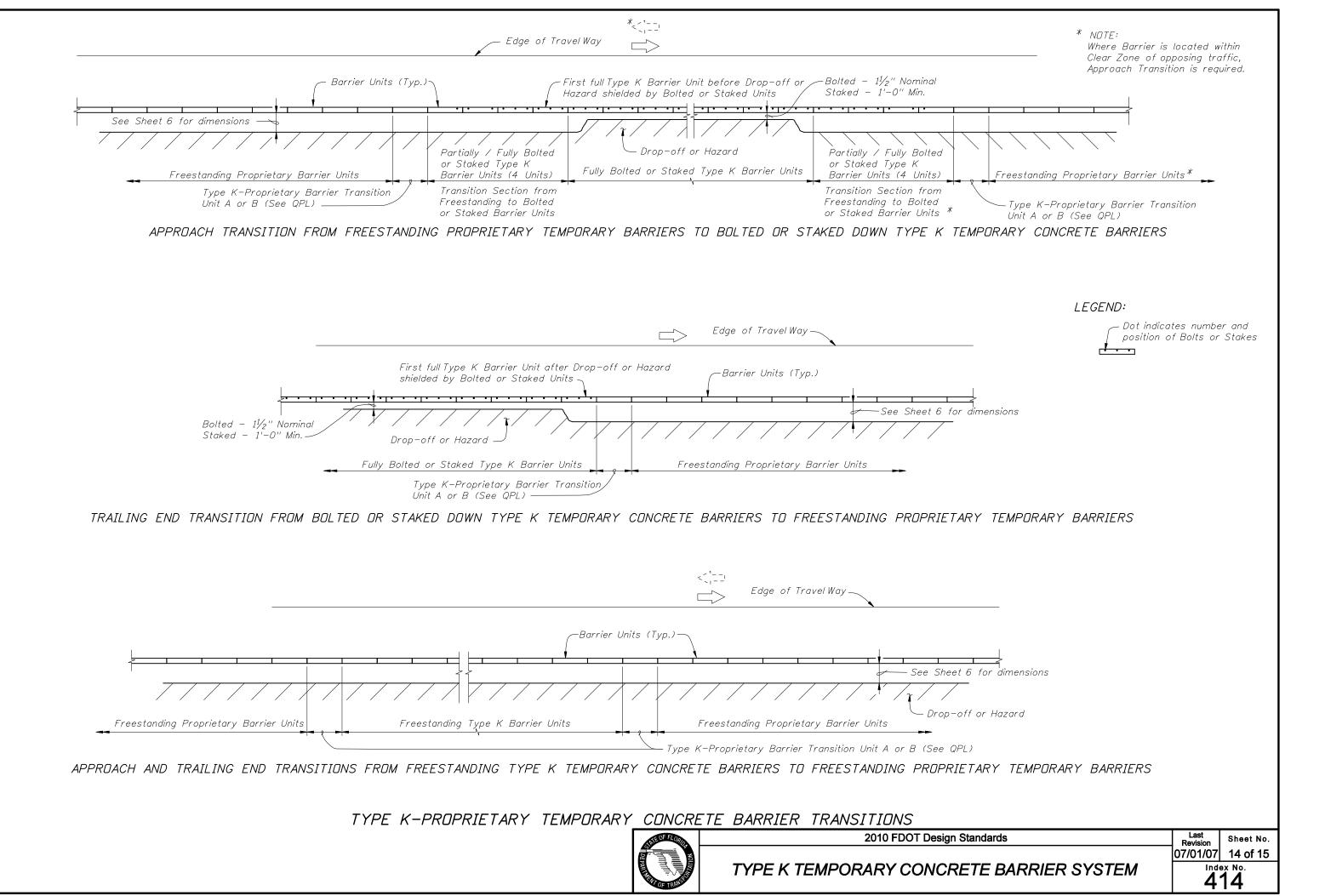
SECTION E-E 32" New Jersey Shape Traffic Railing (shown), 32" F Shape Traffic Railing (similar)



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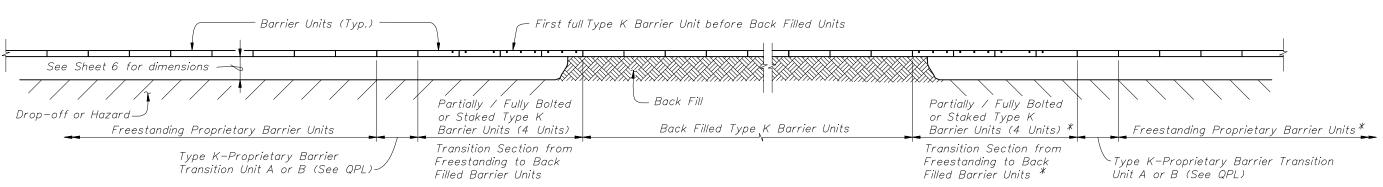
Last Sheet No. 07/01/07 13 of 15

Index No. 414



Where Barrier is located within Clear Zone of opposing traffic, Approach Transition is required.

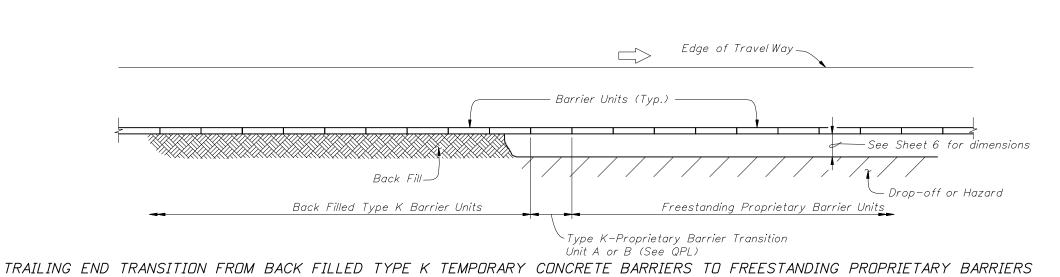


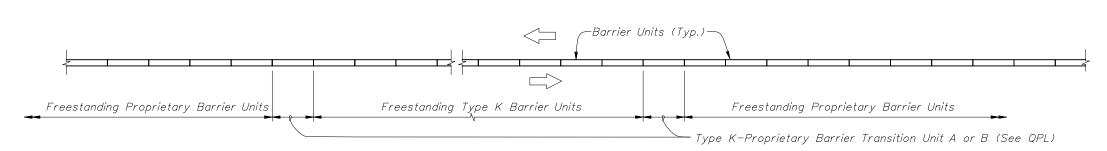


APPROACH TRANSITION FROM FREESTANDING PROPRIETARY TEMPORARY BARRIERS TO BACK FILLED TYPE K TEMPORARY CONCRETE BARRIERS

# LEGEND:

Dot indicates number and position of Bolts or Stakes

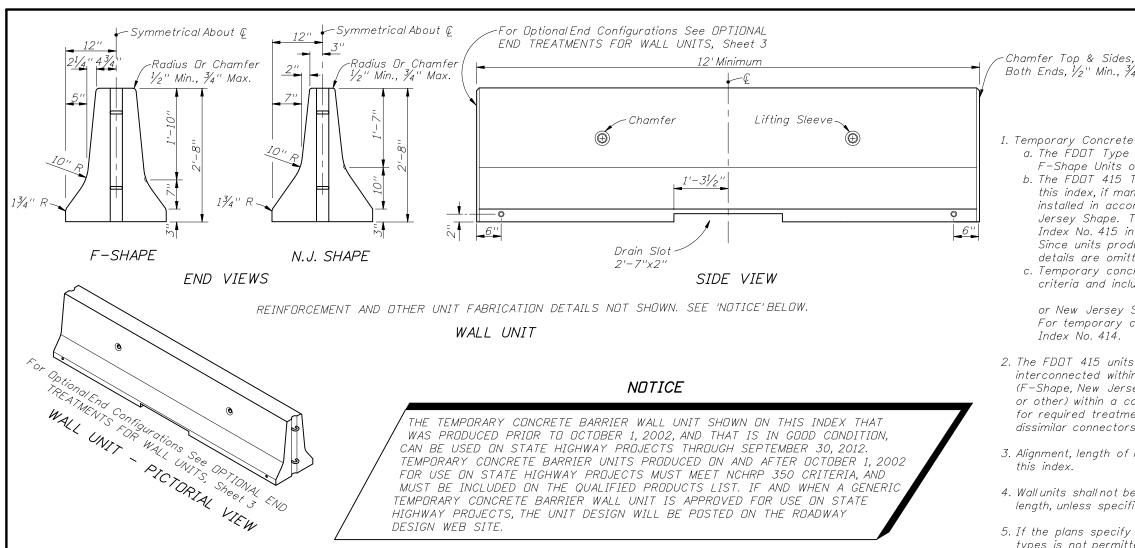




MEDIAN APPROACH AND TRAILING END TRANSITIONS FROM FREESTANDING TYPE K TEMPORARY CONCRETE BARRIERS TO FREESTANDING PROPRIETARY TEMPORARY BARRIERS







DESIGN WEB SITE.

# FDOT 415 TEMPORARY CONCRETE BARRIER WALL UNIT AND GENERAL NOTES

HIGHWAY PROJECTS, THE UNIT DESIGN WILL BE POSTED ON THE ROADWAY

#### When Shielding Above Ground Hazards:

Design Speed	Deflection Space
45 mph or Less	2'
50 mph and Greater	4'

# When Shielding Dropoffs:

Design Speed	Deflection Space
45 mph or Less	2'
50 mph and Greater a. Dropoffs 4' or Less and No Traffic Below b. All dropoff conditions other than 'a'.	2' 4'

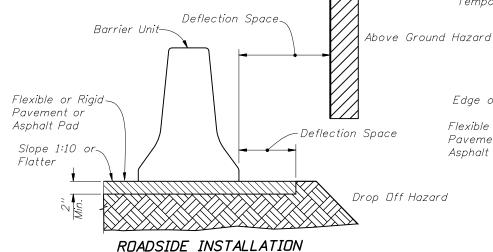
# When used as a Temporary Median Barrier separating opposing traffic lanes:

Design Speed	Offset To Travelway
45 mph or Less	0' min., 2' preferred
50 mph and Greater	2'

Note: These deflection space requirements also apply to approved options identified in General Note 1.

DEFLECTION SPACE REQUIREMENTS

Note: Where existing pavement is not present, construct the Asphalt Pad using Miscellaneous Asphalt Pavement. Cost of the Asphalt Pad to be included in the cost of the Barrier.



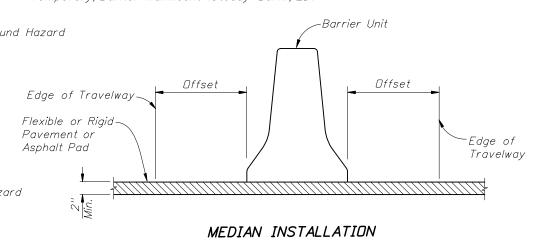
Both Ends, 1/2" Min., 3/4" Max.

#### GENERAL NOTES

- 1. Temporary Concrete Barrier walls on roadways may be any of the following: a. The FDOT Type K Temporary Concrete Barrier Wall (Design Standard Index 414). F-Shape Units only.
  - b. The FDOT 415 Temporary Concrete Barrier wall unit shown on Sheets 1 and 3 of this index, if manufactured prior to October 1, 2002, in good condition, and installed in accordance with this Index. Units may be either F-Shape or New Jersey Shape. The FDOT 415 unit shown in this Index is the design provided in Index No. 415 in prior editions of the Design Standards. See "NOTICE" below. Since units produced after October 1, 2002 cannot be used, complete fabrication details are omitted in this edition of the Design Standards.
  - c. Temporary concrete barrier wall systems meeting NCHRP 350 Test Level 3 criteria and included on the Qualified Products List. Units may be either F-Shape

or New Jersey Shape unless otherwise noted in the plans. For temporary concrete barrier walls on bridges see Design Standard Index No. 414.

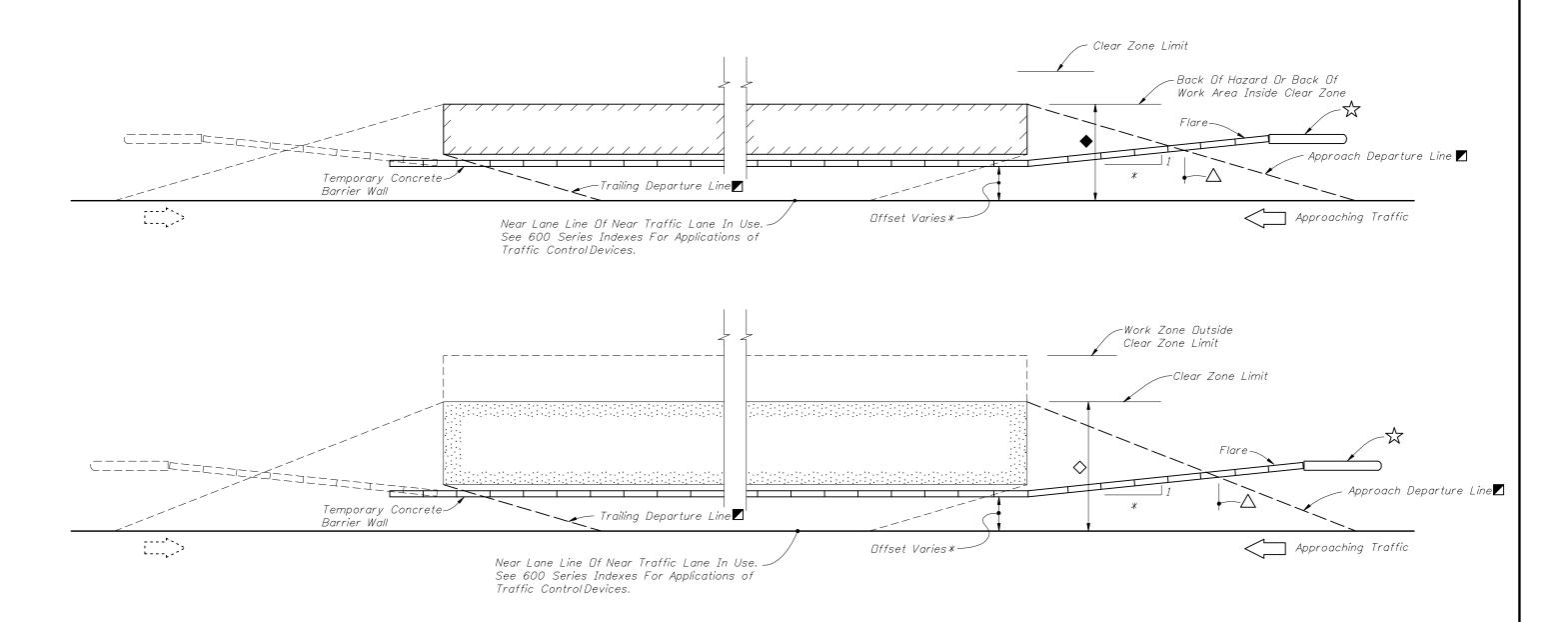
- 2. The FDOT 415 units with the optional end connections shown in this index may be interconnected within a run of wall. However, intermixing units with different shapes (F-Shape, New Jersey Shape) and units with dissimilar end connections (415, Type K, or other) within a continuous run of wall is not permitted. See Sheets 6 through 8 for required treatment for continuation of runs of barrier with different shapes or dissimilar connectors.
- 3. Alignment, length of need, anchorage and end treatment shall be in accordance with
- 4. Wall units shall not be used for permanent barrier wall construction regardless of unit length, unless specifically permitted by the plans.
- 5. If the plans specify Barrier Wall (Temporary) (Type K), substitution with other barrier types is not permitted.
- 6. If the plans specify temporary concrete barrier wall, substitution with water filled barriers is not permitted.
- 7. Type C Steady-Burn Lights are to be mounted on top of temporary concrete barrier walls that are used as barriers along traveled ways in work zones. The lights are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways. For additional information refer to Index 600.
- 8. Wall units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier Wall (Temporary), LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights, Temporary, Barrier Wall Mount (Steady-Burn), ED.



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TEMPORARY CONCRETE BARRIER



△ The approach departure line location is determined by the line intersect with the back of the hazard or the area to be shielded, however the intersect offset distance is not to be beyond the clear zone limit. The trailing departure line is determined by the line intersect with the front of the downstream end of the hazard or the area to be shielded.

The length of barrier wall need is the distance from the approach departure line intersect with the upstream toe of the temporary concrete barrier wall to the trailing departure line intersect with the downstream toe of the temporary concrete barrier wall.

Where temporary concrete barrier wall end units are not anchored, two and one-half  $(2\frac{1}{2})$  wall units (min.) are required beyond the length of barrier need for wall end anchorage. Temporary concrete barrier wall end units shall be located at or outside the clear zone or shielded by other structure, earth embedment or a crash cushion.

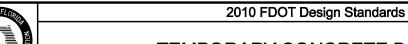
Proprietary redirective crash cushions designed for use with temporary concrete barriers have the beginning length of need and departure line intersect point indicated on the respective QPL drawing for each proprietary crash cushion. Where redirective crash cushions are located on the departure line by their length of need reference point, the wall upstream end unit must be aligned with the crash cushion, and the wall's end unit secured with the anchor plates shown on Sheet 4 of this index. See Sheets 5 through 8 for configurations requiring end unit anchorage.

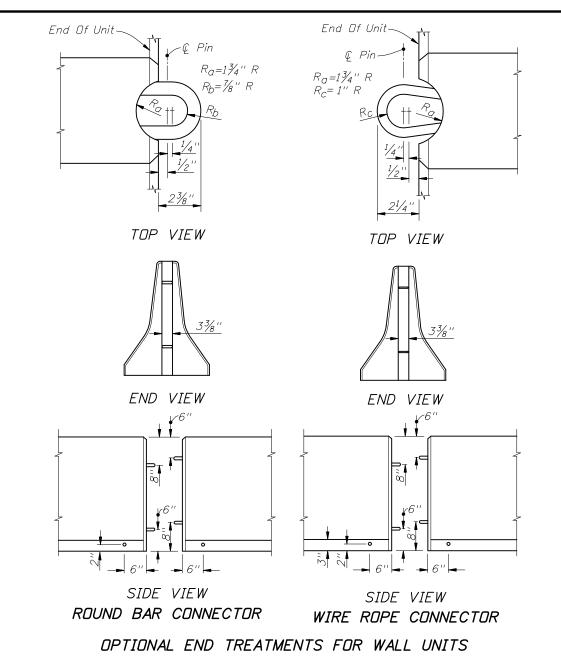
\* The wall offset from the near traffic lane, wall flare rate and wall flare length are to be in conformance with the alignment called for in the plans and the alignments called for by Department Design Standards specified in the plans; in absence of either plan requirement, the offset shall be as determined by the Engineer, and, unless other flare rates are approved by the Engineer the flare rates to be applied are 1 : 10 or flatter for speeds ≤ 45 mph and 1 : 15 or flatter for speeds ≥ 50 mph; see Index No. 642 for other flare rates on freeway facilities.

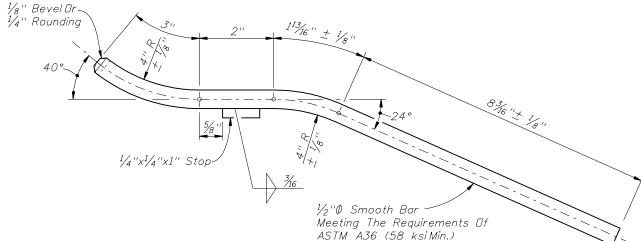
The surface cross slope approaching the barrier wall and continuing across the required deflection space shall not exceed a rate of 1 vertical: 10 horizontal.

# ALIGNMENT AND LENGTH OF NEED

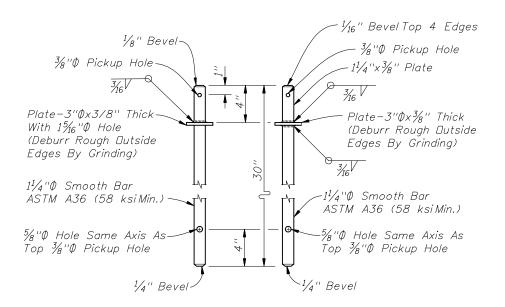
- Departure Rates 1:16 For Speeds ≤ 45 mph 1:13 For Speeds ≥ 50 mph
- Area Shielded When Work Zone Hazards Dr The Work Area Occupy Space Less Than Clear Zone Width
- Area Shielded When Work Zone Hazards
  Or The Work Area Extend To Or Beyond
  Clear Zone Limit
  - Crash Cushion In Absence Of Other Wall End Shielding. See △ Notations And Sheet 5 Through 8 For Varied Locations For Wall End Units And Crash Cushions.



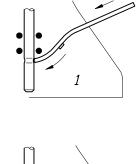


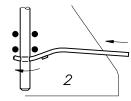


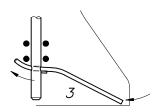
FDOT SNAKE PIN



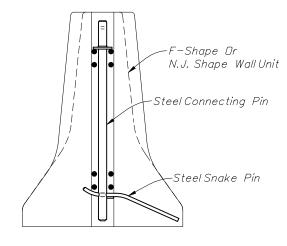
OPTIONAL PINS STEEL CONNECTING PIN

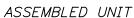


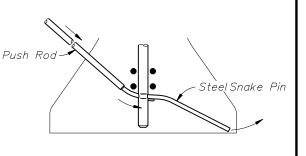




INSERTING FDOT SNAKE PIN



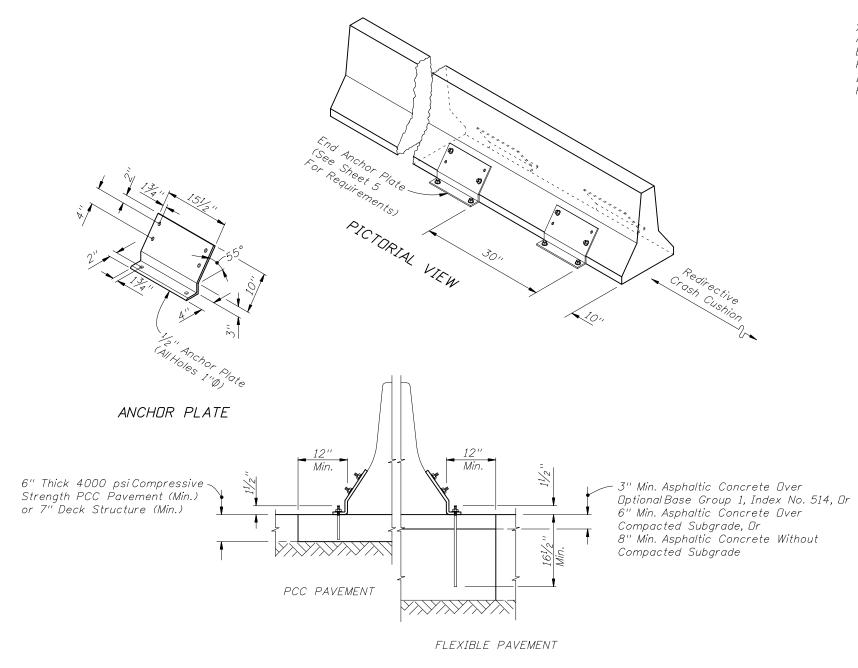




REMOVING FOOT SNAKE PIN

CONNECTING PIN ASSEMBLY





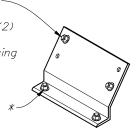
#### SURFACE ANCHORAGE REQUIREMENTS

#### ANCHOR PLATE NOTES

- 1. For temporary barrier wallend units requiring anchor plates, see sheets 5 through 8.
- 2. The temporary concrete barrier wall anchor plate depicted above is a proprietary design by Energy Absorption Systems, Inc.

  Other temporary anchorage methods can be substituted when wall rigidity is assured by any of the following:
  - (a) proven by associated crash test of redirective crash cushions, or
  - (b) meet anchorage prescribed in 'A Guide To Standardized Highway Barrier Hardware', or
  - (c) crash cushion manufacturer's engineered design, or
  - (d) approved shop drawings on a case by case basis.
- 3. The cost for anchoring the wall segment will be included in the cost for the adjoining redirective crash cushion.

3/4"\$\Phix61/2" Adhesive Bonded
Anchor Bolts (EAS MP-3 Or
Equal), 5" Embedment, Two (2)
Required Each Anchor Plate
Installed In Diagonally Opposing
Holes



\* ¾"(\$x6½" Adhesive Bonded Anchor Bolts (EAS MP-3 Dr Equal), 5" Embedment Where Installed On Concrete Pavement Or Decking, Two (2) Required Each Anchor Plate. ¾"(\$x18" MP-3 Threaded Rod Longbolt System Or Other Approved ¾"(\$x18" Threaded Rod With Chemical Anchorage Full Embedment Depth Where Installed On Asphaltic Concrete Pavement Prescribed Below, Two (2) Required Each Anchor Plate.

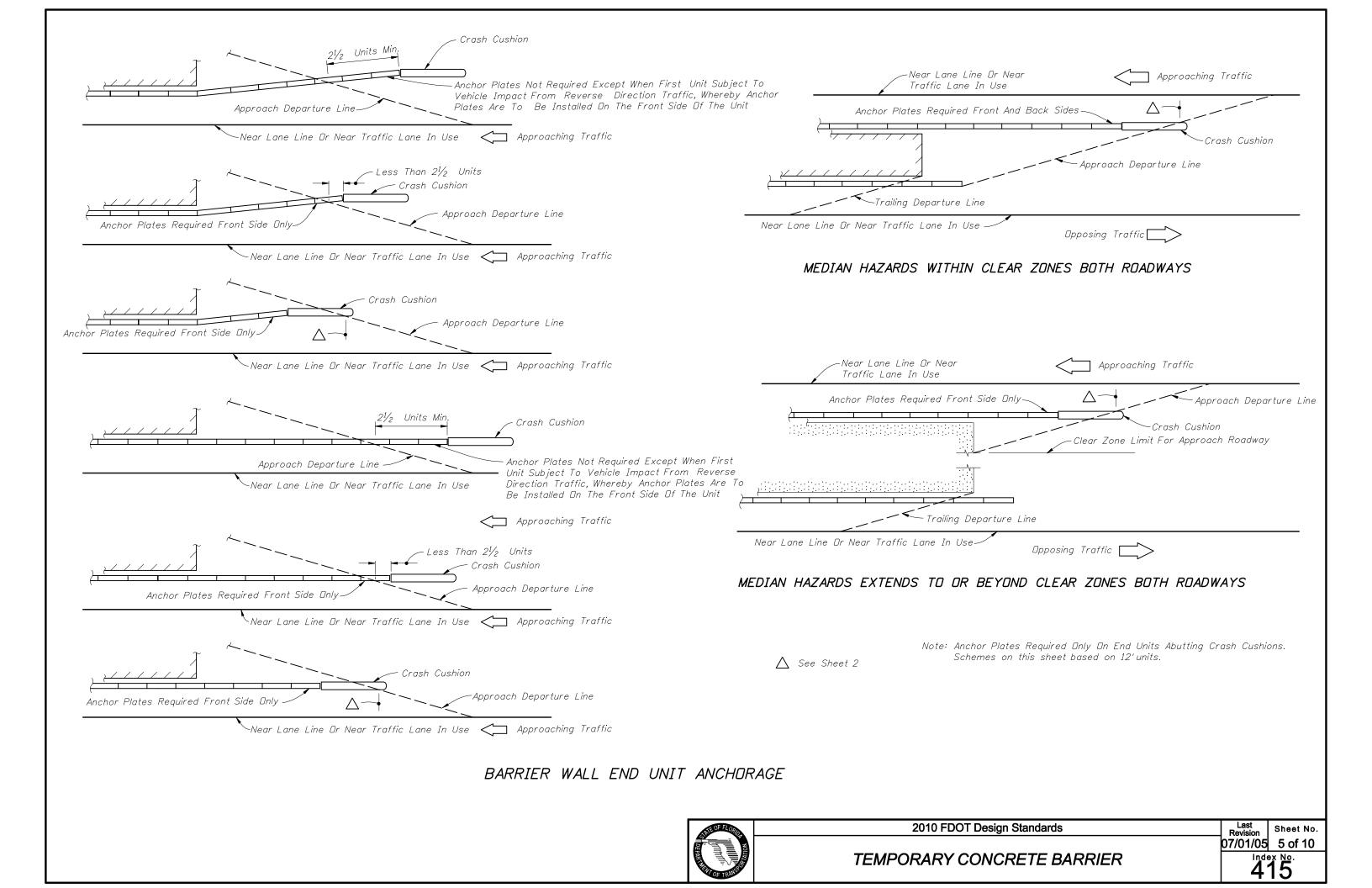
#### ANCHOR PLATE BOLTS

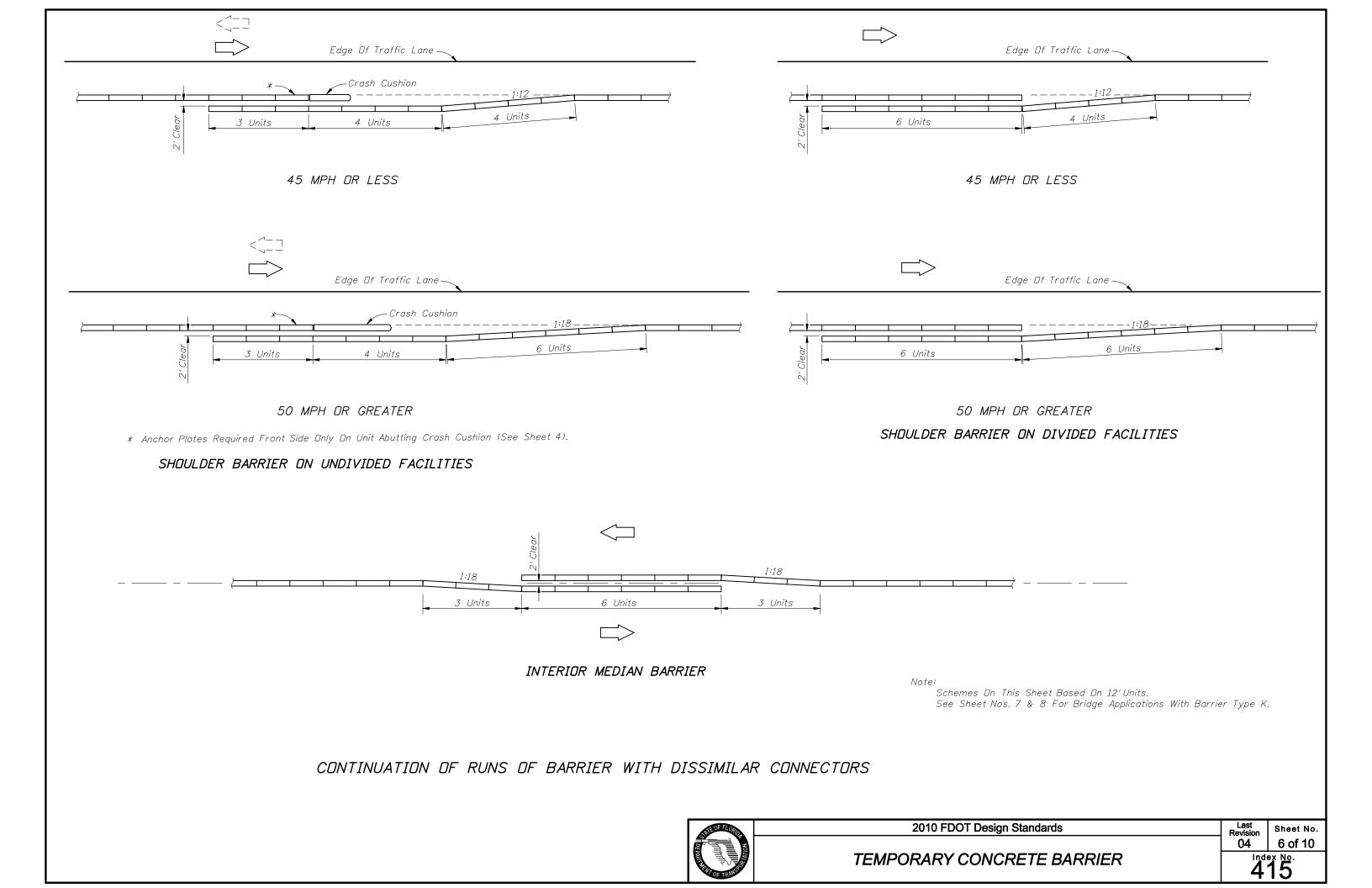
#### NOTES FOR WALL END SHIELDING

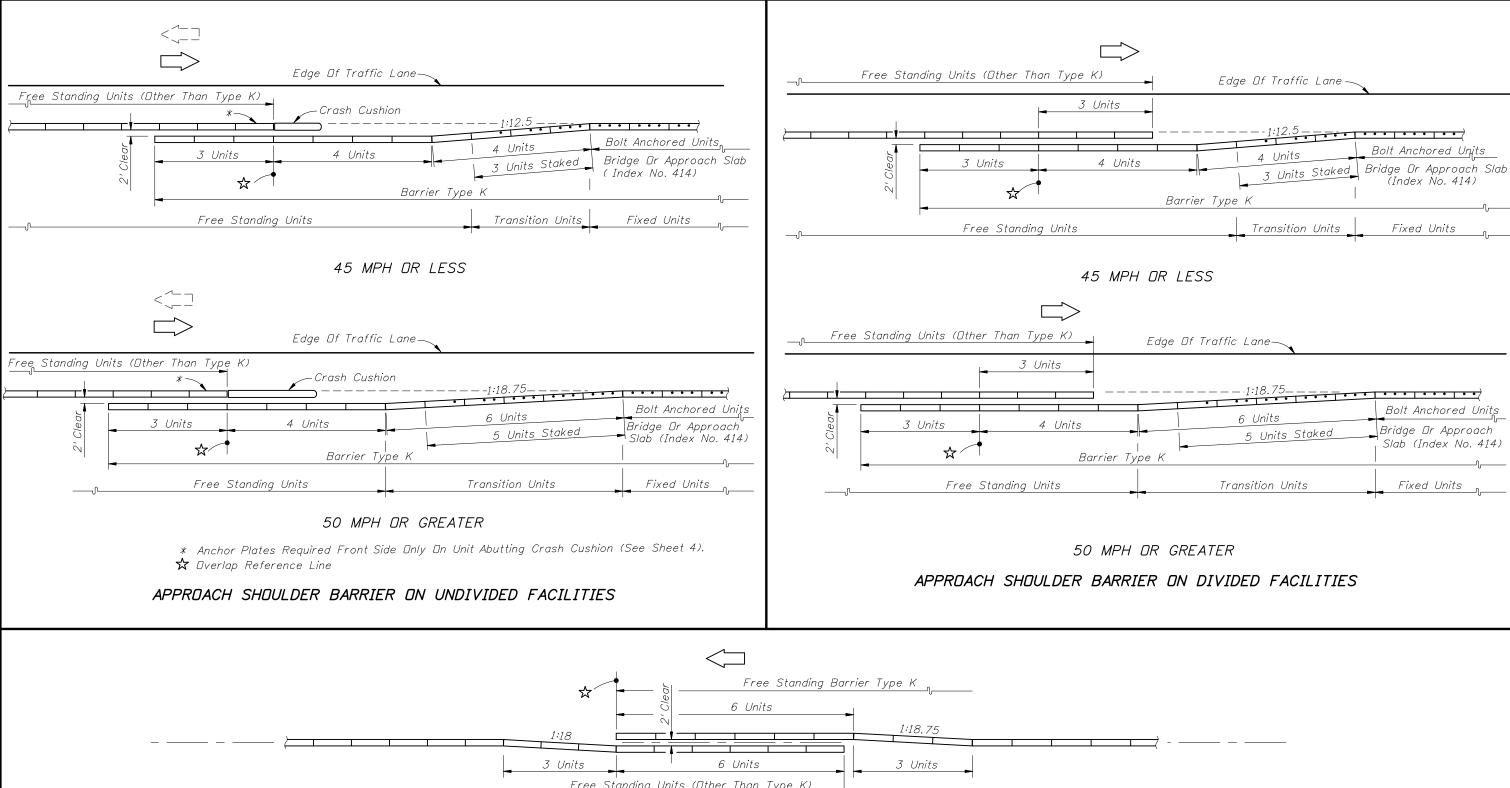
- 1. Redirective crash cushions are the principal (standard) device to be used for shielding approach ends of temporary concrete barrier walls. Except where the plans designate a particular type crash cushion for a specific location, the contractor has the option to construct any of the redirective crash cushions listed on the Qualified Products List, subject to the uses and limitations described on their respective drawings. The barrier wall end unit must be anchored to a paved surface using anchor plates in accordance with "Anchor Plate Notes" and the details on this sheet.
- 2. Temporary redirective crash cushions shall be installed in accordance with the manufacturer's specifications and recommendations. Temporary crash cushions can be either new or functionally sound used devices. Performance of intended function is the only condition for acceptance, whether the crash cushion is new, used, refurbished, purchased, leased, rented, on loan, shared between projects, or made up of mixed new and used components.
- 3. Inertial crash cushions are not optional systems for locations designated for redirective crash cushions by the plans; can not be substituted for redirective crash cushions, and are not eligible for VECP consideration.
- 4. A yellow post mounted Type 1 Object Marker shall be centered 3' in front of the nose of all temporary crash cushions. Mounting hardware shall be in accordance with Index Nos. 11860 and 11865. The cost of the Object Marker shall be included in the cost of the crash cushion.
- 5. Optional temporary redirective crash cushions are to be paid for per location under the contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

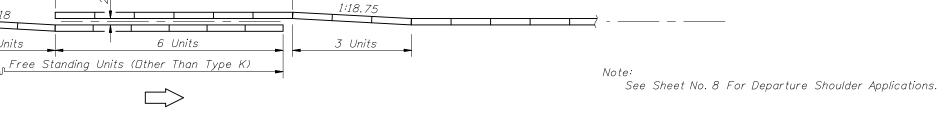
# ANCHOR PLATE REQUIREMENTS FOR BARRIER WALL END UNITS ABUTTING CRASH CUSHIONS









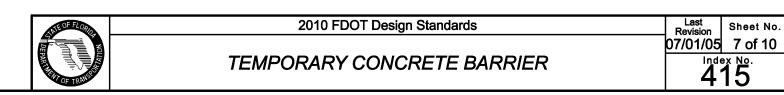


#### INTERIOR MEDIAN BARRIER

LEGEND

Dot Indicates Number Of Bolt Anchors Or Stakes

> CONTINUATION OF BARRIER • FROM OTHER TYPE BARRIERS TO BARRIER TYPE K BARRIER TYPE K ON BRIDGES AND APPROACH SLABS

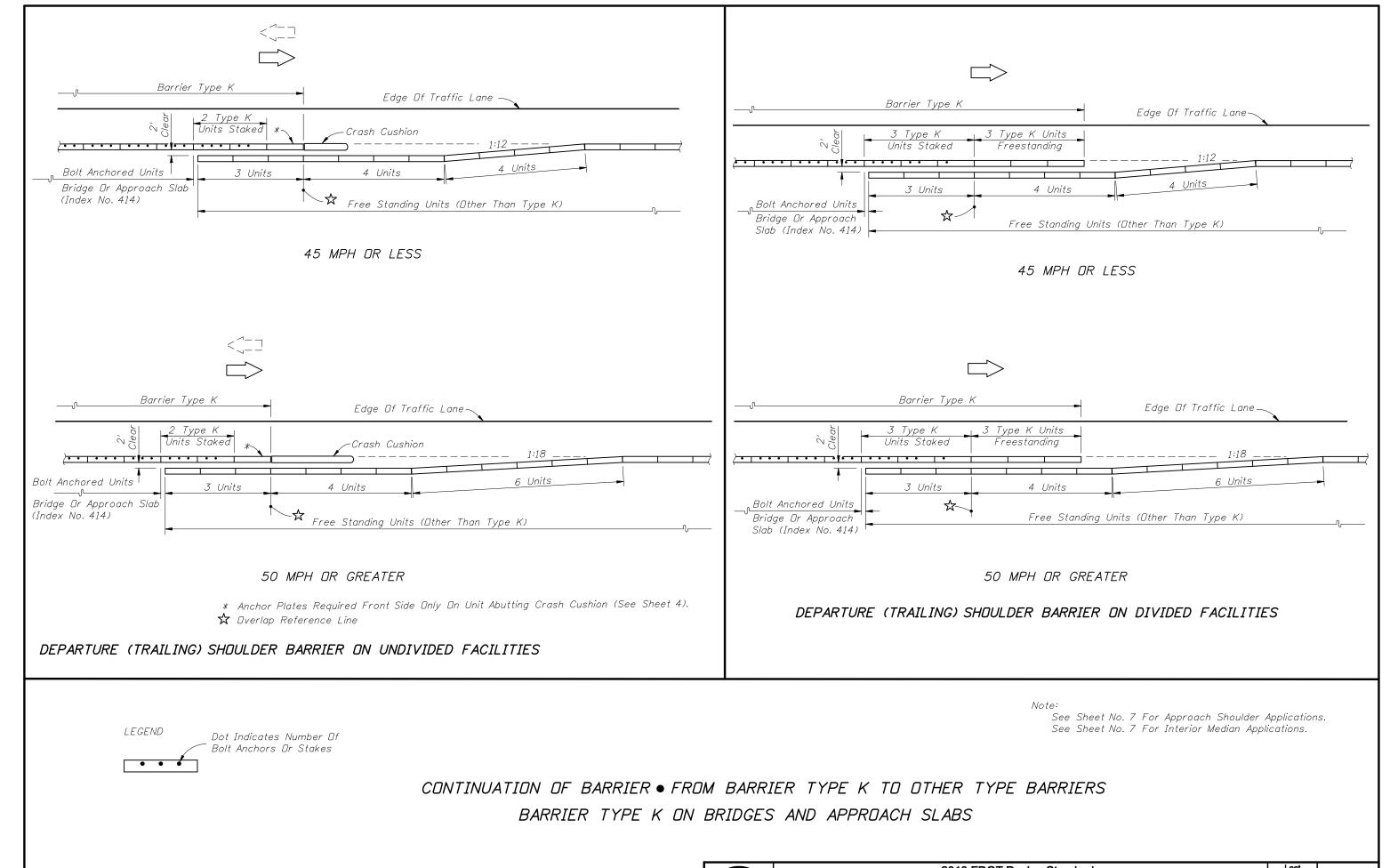


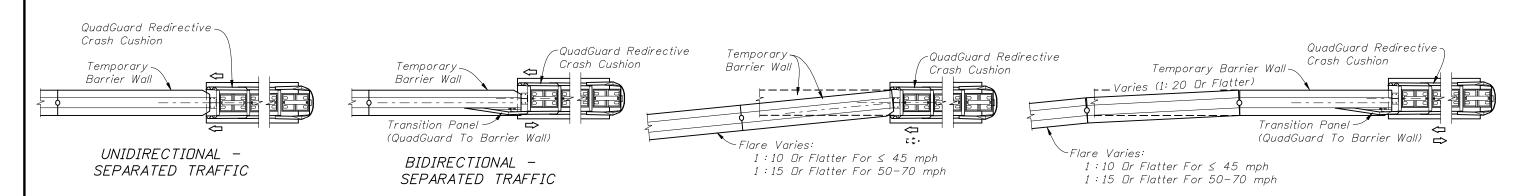
Fixed Units

Bolt Anchored Units

Slab (Index No. 414)

Fixed Units



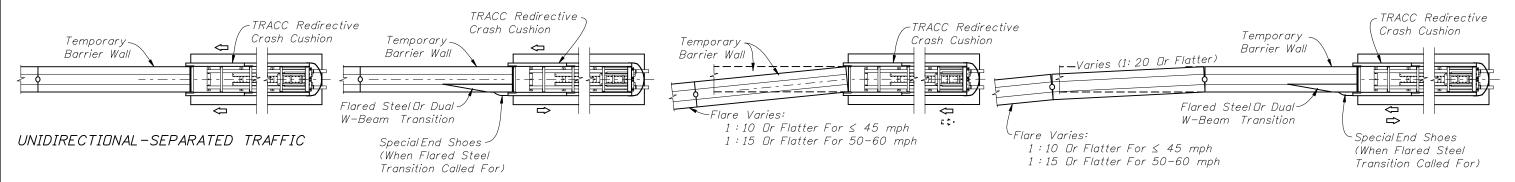


TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED OUTSIDE OPPOSING LANE CLEAR ZONE OR ONE-WAY TRAFFIC

TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED WITHIN OPPOSING LANE CLEAR ZONE

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)

# WALL END TREATMENT WHEN SHIELDED BY A QuadGuard CRASH CUSHION



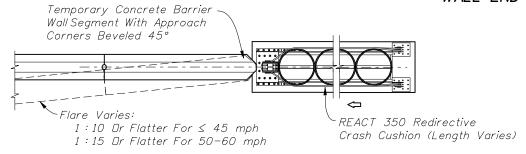
BIDIRECTIONAL-SEPARATED TRAFFIC

TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED OUTSIDE OPPOSING LANE CLEAR ZONE OR ONE-WAY TRAFFIC

TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED WITHIN OPPOSING LANE CLEAR ZONE

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)

#### WALL END TREATMENT WHEN SHIELDED BY A TRACC CRASH CUSHION



FOR ANY APPROACH CONDITION IT SHALL BE IN ACCORDANCE WITH THE DRAWINGS POSTED ON THE QUALIFIED PRODUCTS LIST

WALL END TREATMENT WHEN SHIELDED BY A REACT 350 CRASH CUSHION

#### NOTES

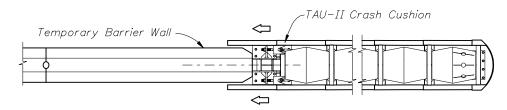
- 1. For alignment and length of need see Sheets 2 and 5 through 8.
- 2. Anchor plates required only on units abutting crash cushions.
- 3. For crash cushion details see drawings posted on the Qualified Products List at "544 Vehicle Impact Attenuators".

# SHIELDING WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION) (CONTINUATION ON SHEET 10)

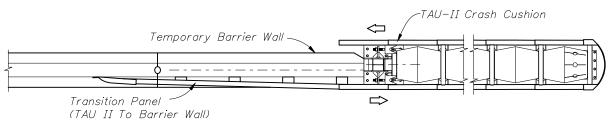


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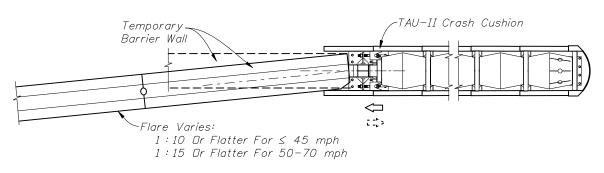
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UNIDIRECTIONAL - SEPARATED TRAFFIC

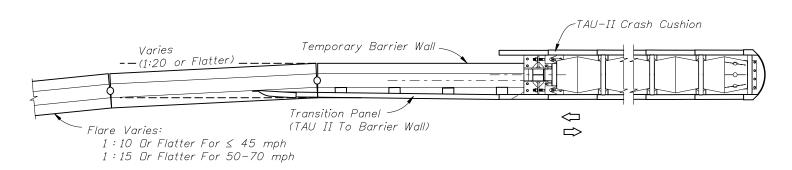


BIDIRECTIONAL - SEPARATED TRAFFIC



TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED DUTSIDE OPPOSING LANE CLEAR ZONE OR ONE-WAY TRAFFIC

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)



TWO-WAY TRAFFIC WITH CRASH CUSHION LOCATED WITHIN OPPOSING LANE CLEAR ZONE

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)

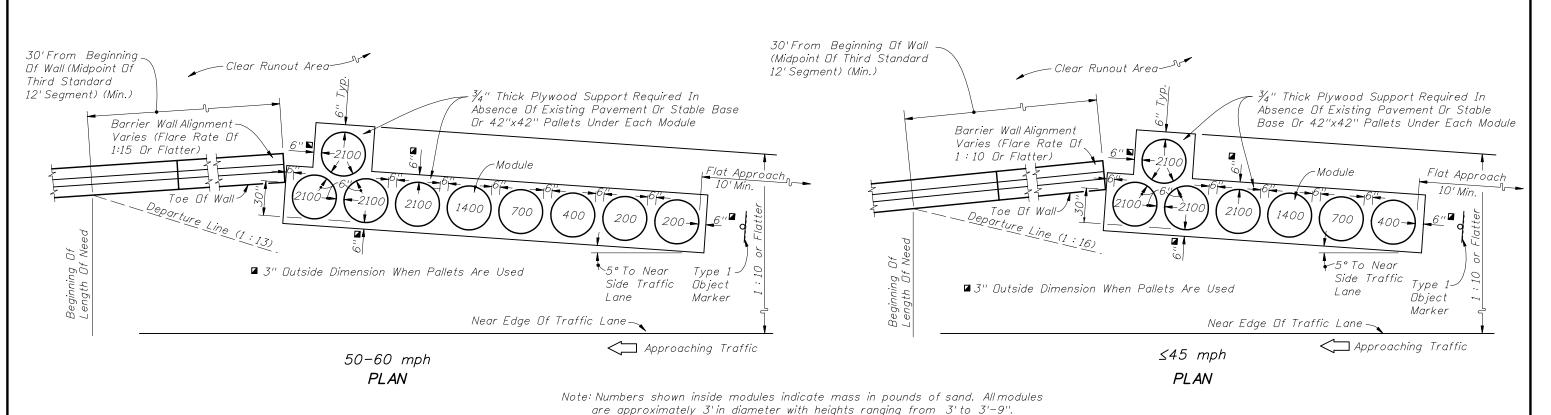
# WALL END TREATMENT WHEN SHIELDED BY TAU II CRASH CUSHION

#### NOTES

- 1. For alignment and length of need see Sheets 2 and 5 through 8.
- 2. Anchor plates required only on units abutting crash cushions.
- 3. For crash cushion details see drawings posted on the Qualified Products List.

# SHIELDING WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION)





INERTIAL CRASH CUSHION ARRAYS

# 2"x2"x22" Symmetrically Spaced Cleats On Corners For 3' 0

Pallet Shall Be Constructed Of Wood Or Other Frangible Or Resilient Materials Other Than Metals, And, Shall Be Sufficiently Durable To Support Modules For Their Expected Period Of Use; Wood Pallet Detail Shown.

# INERTIAL MODULE PALLET

Module Or Other Retainer

As Approved By The Engineer

#### NOTES FOR TEMPORARY GATING CRASH CUSHIONS

- 1. The crash cushion arrays shown on this Index can be used on the State Highway System only when all of the following conditions are met:

  (a) Use is limited to shielding temporary concrete barrier wall approach ends.
- (b) Used only when a temporary gating crash cushion or inertial crash cushion is specifically called for in the plans.
- (c) Use is limited to installations that will not exceed 30 calendar days in duration, unless otherwise called for in the plans.

When the plans do not specifically call for a temporary gating crash cushion, and/or when the installation will exceed 30 days in duration, a redirective crash cushion system in accordance with Index No. 415 is required.

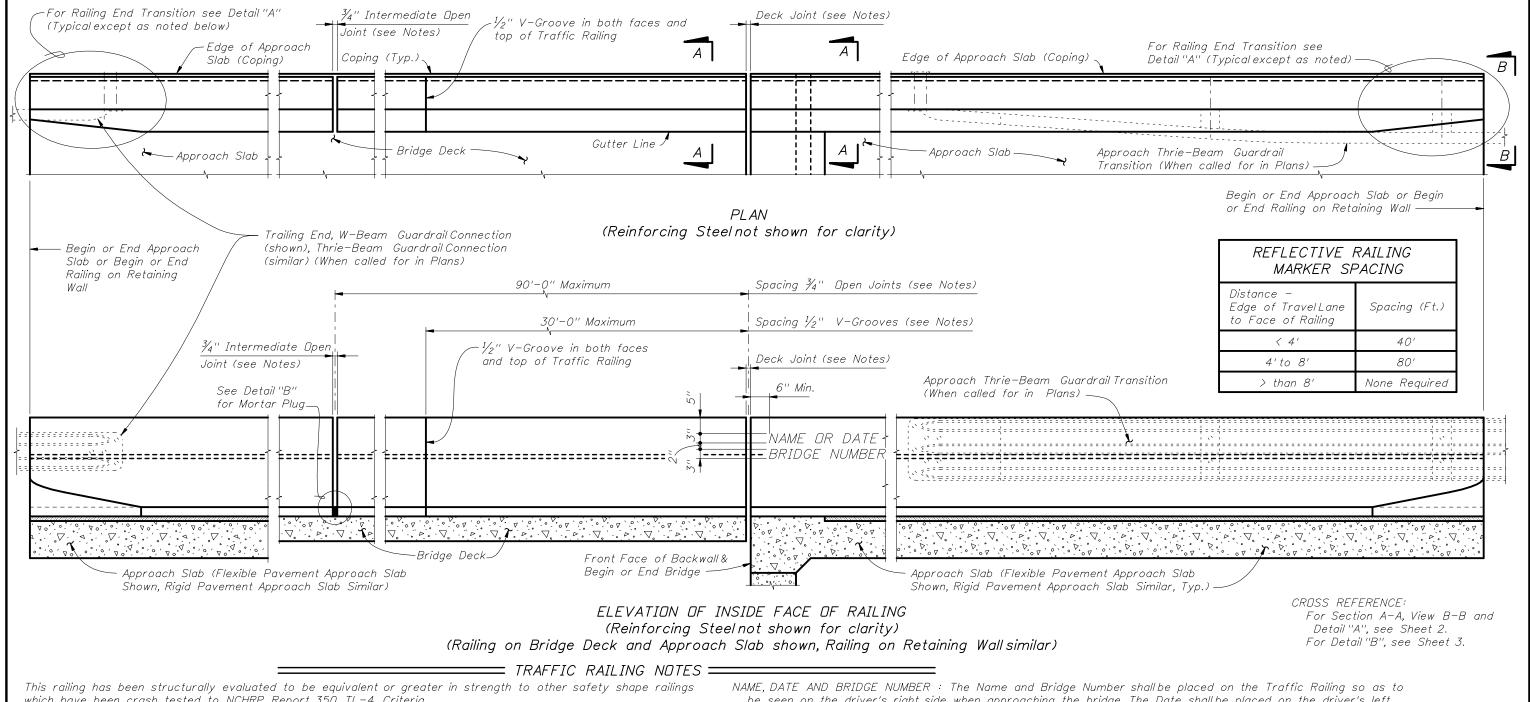
- 2. Inertial crash cushions are gating type crash cushions, and a clear runout area back of the array must be provided. The arrays shown can be used for outer roadway applications, exclusive of gore areas, and for median applications where the median width is sufficient to provide clear zone width between the back side module and the near lane of the opposing traffic.
- 3. Inertial crash cushion modules shall be installed in accordance with the manufacturer's specifications and recommendations, and can be constructed of either new or functionally sound used modules.
- 4. Anchorage of barrier wallend segment is not required.
- 5. A yellow post mounted Type I Object Marker shall be centered 3' in front of the nose of all crash cushion arrays. Mounting hardware shall be in accordance with Index Nos. 11860 and 11865. The cost of the Object Marker shall be included in the cost of the crash cushion.
- 6. Temporary gating crash cushion systems listed on the Department's Qualified Products List (QPL) may be substituted for the crash cushion arrays shown in this Index, provided a configuration using the system for this substitution has been detailed in the approved QPL drawings. Manufacturers seeking approval of temporary gating crash cushions for inclusion on the QPL must submit application along with design documentation showing the crash cushion system is crash tested to NCHRP Report 350 Test Level 3 criteria, is accepted by FHWA and is compatible with FDDT temporary barrier wall systems. System approvals will be contingent on FDDT's evaluation of crash test performance results for consistency with FDDT temporary barrier wall end shielding applications and uses. If approved, installation drawings signed and sealed by a professional engineer licensed in the State of Florida will be required.
- 7. Temporary crash cushions (gating) are to be paid for, per array, under the contract unit price for Vehicular Impact Attenuator/Crash Cushion (Gating) (Temporary), LO.

# TEMPORARY INERTIAL CRASH CUSHIONS FOR SHIELDING ENDS OF TEMPORARY CONCRETE BARRIER WALL



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which have been crash tested to NCHRP Report 350 TL-4 Criteria.

CONCRETE AND REINFORCING STEEL: See Structures Plans General Notes.

MARKERS: Elevation Markers shall be placed on top of the Traffic Railing at the end bents. On bridges longer than 100 ft. one marker shall be placed at each end of the bridge. On bridges 100 ft. or less one marker shall be placed at one end of the bridge only. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The cost of installing the markers shall be included in the Contract Unit Price for the Traffic Railing.

GUARDRAIL: For Guardrail connection details see Index Nos. 400 and 410.

SUPERELEVATED BRIDGES: At the option of the Contractor the Traffic Railing on superelevated bridges may be constructed perpendicular to the roadway surface. If an adjoining railing is constructed plumb, transition the end of the Traffic Railing from perpendicular to plumb over a minimum distance of 20'-0". The cost of all modifications will be at the Contractor's expense.

PEDESTRIAN AND BICYCLE RAILING: See Index Nos. 821 and 822 for Notes, Details and post spacings for Traffic Railings with Aluminum Pedestrian / Bicycle Bullet Railings.

V-GROUVES: Construct 1/2" V-Grooves plumb. Space V-Grooves equally between 3/4" Open Joints and/or Deck Joints and at V-Groove locations on Retaining Wall footings.

be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Name shall be as shown in the General Notes in the Structures Plans. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the year of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by  $\frac{3}{8}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing.

JOINTS : See Plans, Superstructure, Approach Slab and Retaining Walls Sheets for actual dimensions and joint orientation. Open Railing Joints at Deck Expansion Joint locations shall match the dimensions of the Deck Joint. For treatment of Railings on skewed bridges see Index No. 490. Deck Joint at Begin Bridge or End Bridge shown, Deck Joint at C Pier or Intermediate Bent similar. Provide 3/4" Intermediate Open Joints at:

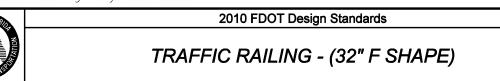
Sheet No.

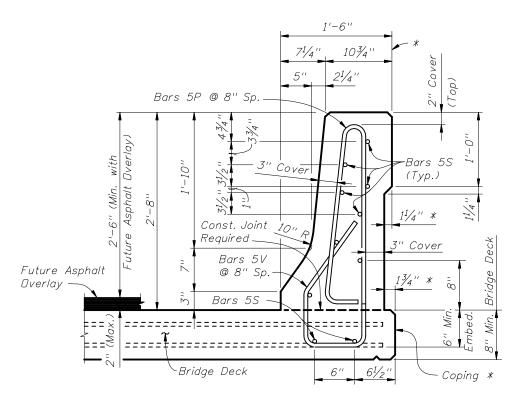
1 of 3

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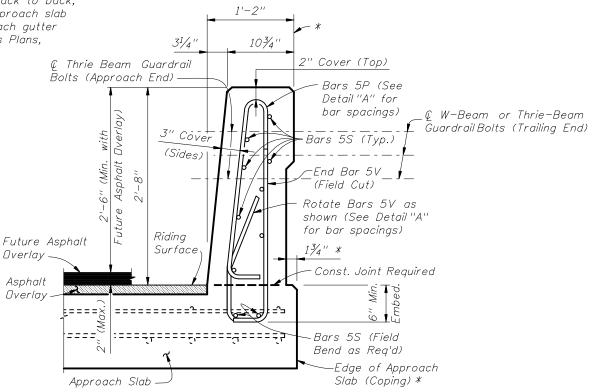
07/01/08

- (1) Substructure supports where superstructure slab is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.
- (4) At ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.



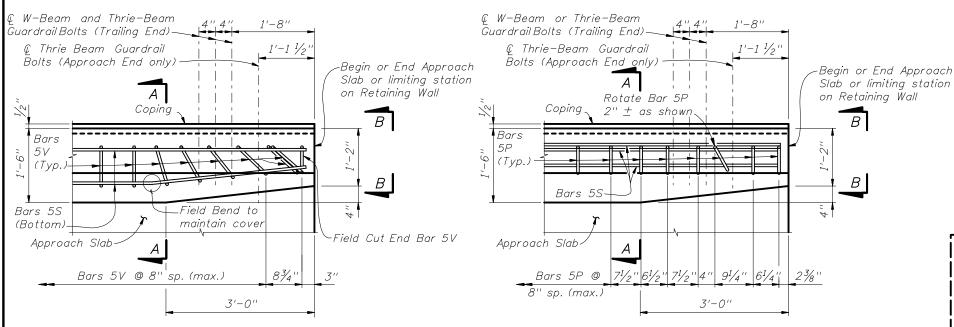


\* Where railings of adjacent bridges are to be built back to back, the outside vertical plane of the railing and deck/approach slab may coincide along a plane centered 1'-6" from each gutter line. A bond breaker will be required. See Structures Plans, Superstructure Sheets for Details.



VIEW B-B (Section thru Approach Slab shown, Section thru Retaining Walls similar)

SECTION A-A
TYPICAL SECTION THRU TRAFFIC RAILING
(Section thru Bridge Deck shown, Section thru Approach Slab
and Retaining Walls similar)



PLAN - Railing End Transition (Showing Bars 5V and 5S) PLAN – Railing End Transition (Showing Bars 5P and 5S)

NDTE: Omit Railing End Transition and Guardrail if Index 410 Concrete Barrier Wall is used beyond the Approach Slab or Retaining Wall.

See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5P and 5V at 8" (Typ.)

#### VUTES:

Rotate Bars 5V in Railing End Transition to maintain cover. Begin placing Railing Bars 5P and 5V on Approach Slab at the barrier end and proceed toward Begin or End Bridge to ensure placement of guardrailbolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.

CROSS REFERENCE:
For locations of Section A-A and
View B-B see Sheet 1.

#### INSTRUCTIONS TO DESIGNER:

For Bridge Decks up to a maximum thickness of 9", the two Bars 5S placed in the Bridge Deck may substitute for the longitudinal deck steel located within the limits of Bars 5V, provided that the total area of longitudinal deck steel beneath the railing, as required by calculation, is not reduced. Show these bars on the Structures Plans, Superstructure Sheets with the deck steel.

All Bars 5P, 5S and 5V as shown are included in the Estimated Traffic Railing Quantities. Do not include Bars 5P, 5S and 5V in the reinforcing bar lists and estimated quantities for supporting bridge decks, approach slabs or retaining walls.

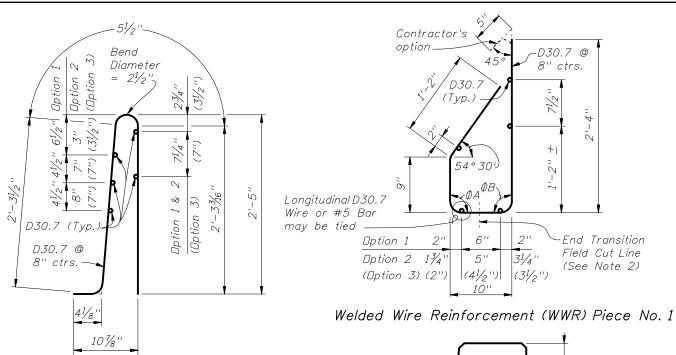


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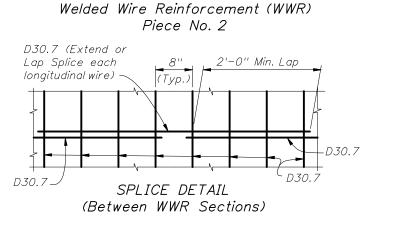
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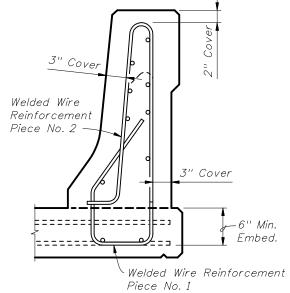
TRAFFIC RAILING - (32" F SHAPE)

Index No. 420



ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS





#### WELDED WIRE REINFORCEMENT NOTES:

- 1. At the option of the Contractor Welded Wire Reinforcement (WWR) may be utilized in lieu of all Bars 5P, 5S and 5V. Welded Wire Reinforcement shall conform to ASTM A497.
- 2. Welded Wire Reinforcement at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The vertical wires (D30.7) in Piece 1 shall be cut as shown and the gutter side portion bent inward as required to allow placement.

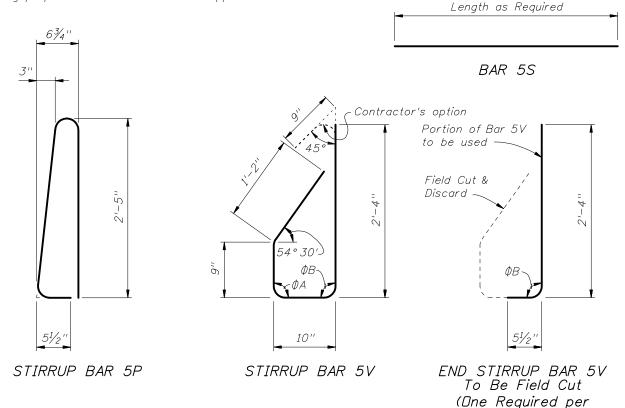
	CONVEI	VTIONAL	REINFO	DRCING	STEEL	BE	NDING	DIA	AGRAMS
	LOW (	GUTTER	HIGH (	GUTTER			BILL	0F	REINFO
LOPE	ΦA	ΦB	ФА	ΦВ			MARK	< <b>│</b>	SIZE

ROADWAY	LOW G	GUTTER	HIGH (	GUTTER
CROSS-SLOPE	ФА	ΦB	ФА	ΦВ
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	93°
6% to 10%	96°	84°	84°	96°

BILL OF REINFORCING STEEL						
MARK	SIZE	LENGTH				
Р	5	5'-7''				
S	5	As Reqd.				
V	5	5'-1''				

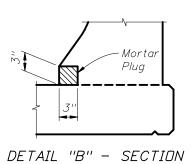
Railing End Transition)

 $\emptyset A$  and  $\emptyset B$  shall be 90° if Contractor elects to place railing perpendicular to the deck and approach slabs.



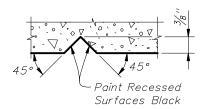
REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The 9" and the 2'-4" vertical dimensions shown for Bar 5V are based on a bridge deck without a raised sidewalk. If a raised sidewalk is to be provided, increase these dimensions to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above for a 8" deck with  $\Phi A = \Phi B = 90^{\circ}$
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".



AT INTERMEDIATE OPEN JOINT

At Intermediate Open Joints, plug the lower 3" portion of the open joint by filling it with mortar in accordance with Section 400 of the Specifications.



End Transition

SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

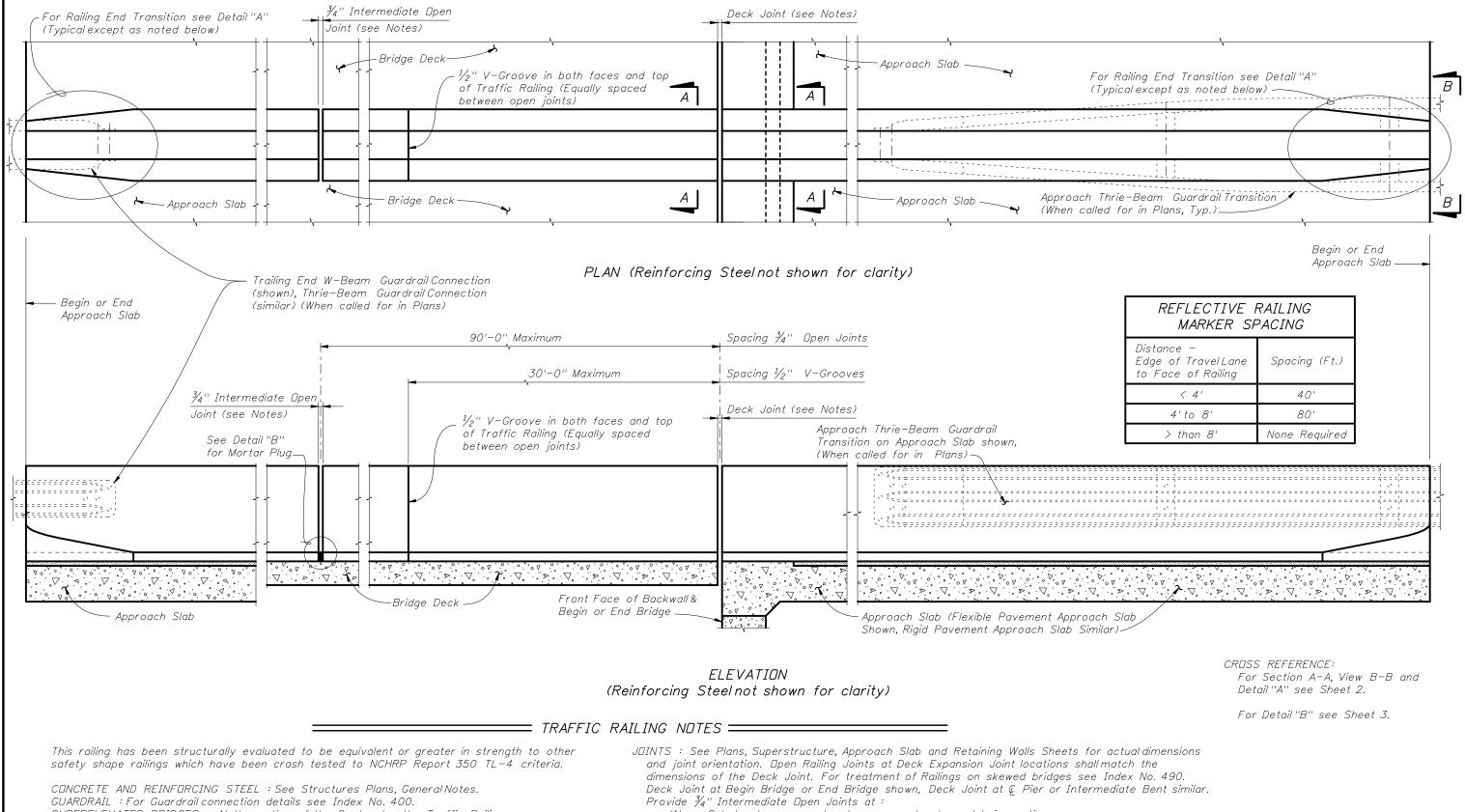
ESTIMATED ( QUA	TRAFFIC NTITIES	RAILING
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.104
Reinforcing Steel	LB/LF	27.12

(The above quantities are based on a 2% deck cross slope; railing on low side of deck.)



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2010	FUUI	Design	Stantianu

Sheet No. 07/01/07 3 of 3 1ndex No. 420



CONCRETE AND REINFORCING STEEL: See Structures Plans, General Notes.

GUARDRAIL: For Guardrail connection details see Index No. 400.

SUPERELEVATED BRIDGES: At the option of the Contractor the Traffic Railing on superelevated bridges may be constructed perpendicular to the roadway surface.

The cost of all modifications will be at the Contractor's expense.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing along the centerline at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing.

- (1) Substructure supports where superstructure slab is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.

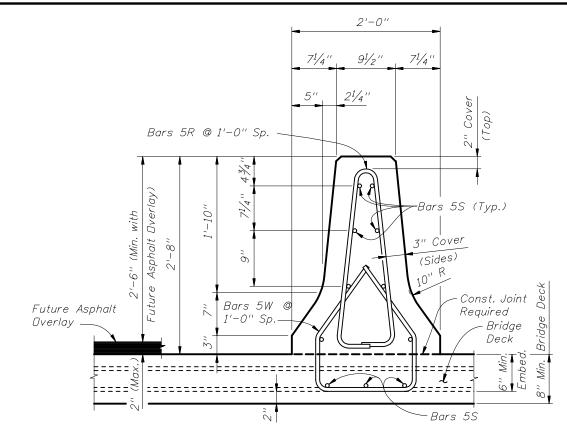
2010 FDOT Design Standards

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TRAFFIC RAILING - (MEDIAN 32" F SHAPE)

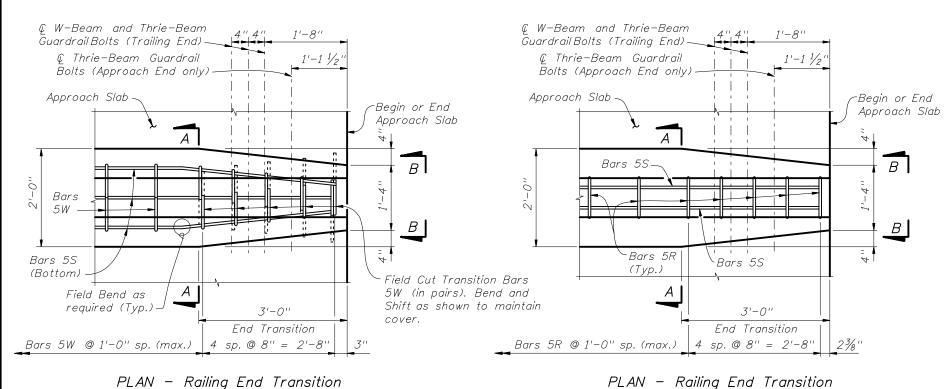
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Index No. 421



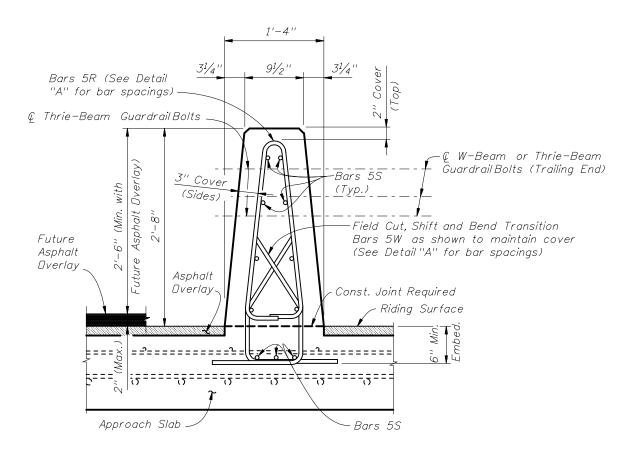
(Showing Bars 5W and 5S)

SECTION A-A
TYPICAL SECTION THRU TRAFFIC RAILING
(SECTION THRU BRIDGE DECK SHOWN SECTION THRU APPROACH SLAB SIMILAR)



DETAIL "A"

PLAN – Railing End Transition (Showing Bars 5R and 5S)



VIEW B-B

#### VOTE:

Begin placing Railing Bars 5R and 5W on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5R and 5W shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5R and 5W as required to maintain cover in Railing End Transition.

Omit Railing End Transition and Guardrailif Index 410 Concrete Barrier Wall is used beyond the Approach Slab. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Transition is omitted, extend Typical Section to the end of Approach Slab and space Bars 5R and 5W at 1'-0" (Typ.)

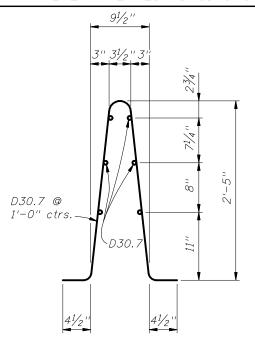
#### INSTRUCTIONS TO DESIGNER:

For Bridge Decks up to a maximum thickness of 9", the three Bars 5S placed in the deck may substitute for the longitudinal deck steel located within the limits of Bars 5W, provided that the total area of longitudinal deck steel beneath the railing, as required by calculation, is not reduced. Show these bars on the Structures Plans, Superstructure Sheets with the deck steel.

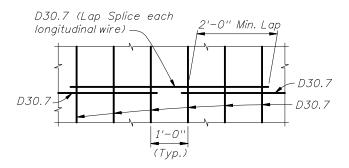
All Bars 5R, 5S and 5W as shown are included in the Estimated Traffic Railing Quantities. Do not include Bars 5R, 5S and 5W in the reinforcing bar lists and estimated quantities for supporting bridge decks or approach slabs.



# ALTERNATE REINFORCING STEEL (WELDED WIRE REINFORCEMENT) DETAILS

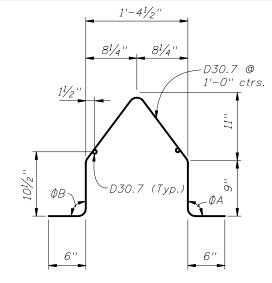


Welded Wire Reinforcement (WWR) Piece No. 2

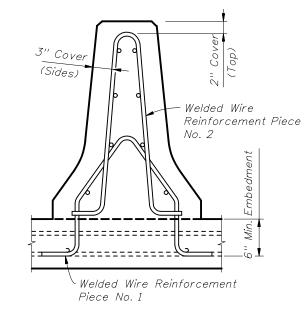


SPLICE DETAIL (Between WWR Sections)

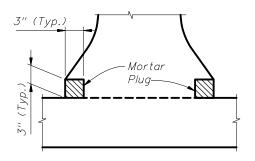
WELDED WIRE REINFORCEMENT NOTES:



Welded Wire Reinforcement (WWR)
Piece No. 1



- 1. At the option of the Contractor Welded Wire Reinforcement may be utilized in lieu of all Bars 5R, 5S and 5W. Welded Wire Reinforcement shall conform to ASTM A497.
- 2. Welded Wire Reinforcement at Railing End Transition shall be field bent inward as required (Pieces 1 & 2) to maintain cover. The top of Piece 1 shall be cut to allow overlap.
- 3. Place WWR panels so as to minimize the end overhang of longitudinal wires at Railing Ends and Open Joints. Overhangs greater than 6" are not permitted.



DETAIL "B" — SECTION AT INTERMEDIATE OPEN JOINT

NOTE: At Intermediate Open Joints, plug the lower 3" portion of the open joint by filling it with mortar in accordance with Section 400 of the Specifications.

#### ON SLOPE AT CROWN ROADWAY CROSS-SLOPE ΦА ΦВ ΦА 90° 90° 90° 90° 0% to 2% 93° 87° 90° 90° 2% to 6% 84° 90° 90° 96° 6% to 10%

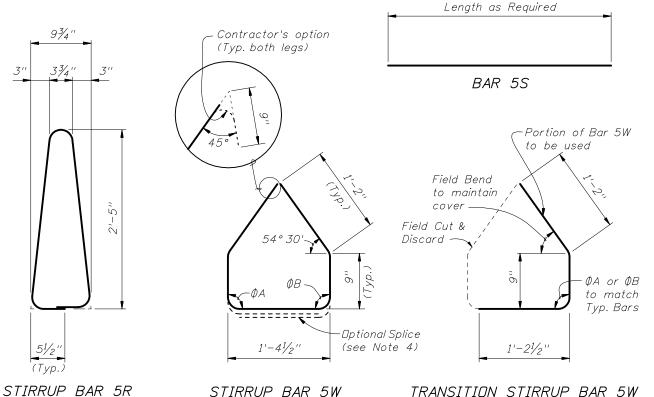
ФΑ	an	d ΦB	shall be	90°	if	Contr	actor	elects	to	place	
raili	ng	perp	endicular	to	the	deck,	and	approad	ch	slabs.	

MARK         SIZE         LENGTH           R         5         6'-1"           S         5         As Reqd.           W         5         5'-3"	BILL OF REINFORCING STEEL						
S 5 As Regd.	MARK	SIZE	LENGTH				
7,5 7,6 90.	R	5	6'-1''				
W 5 5'-3''	S	5	As Reqd.				
	W	5	5'-3"				

To Be Field Cut and Bent

(10 required per Railing

End Transition)



CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 3. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 4. At the Contractor's option, Bars 5W may be fabricated as a two piece bar with a 1'-2'' lap splice of the bottom legs.

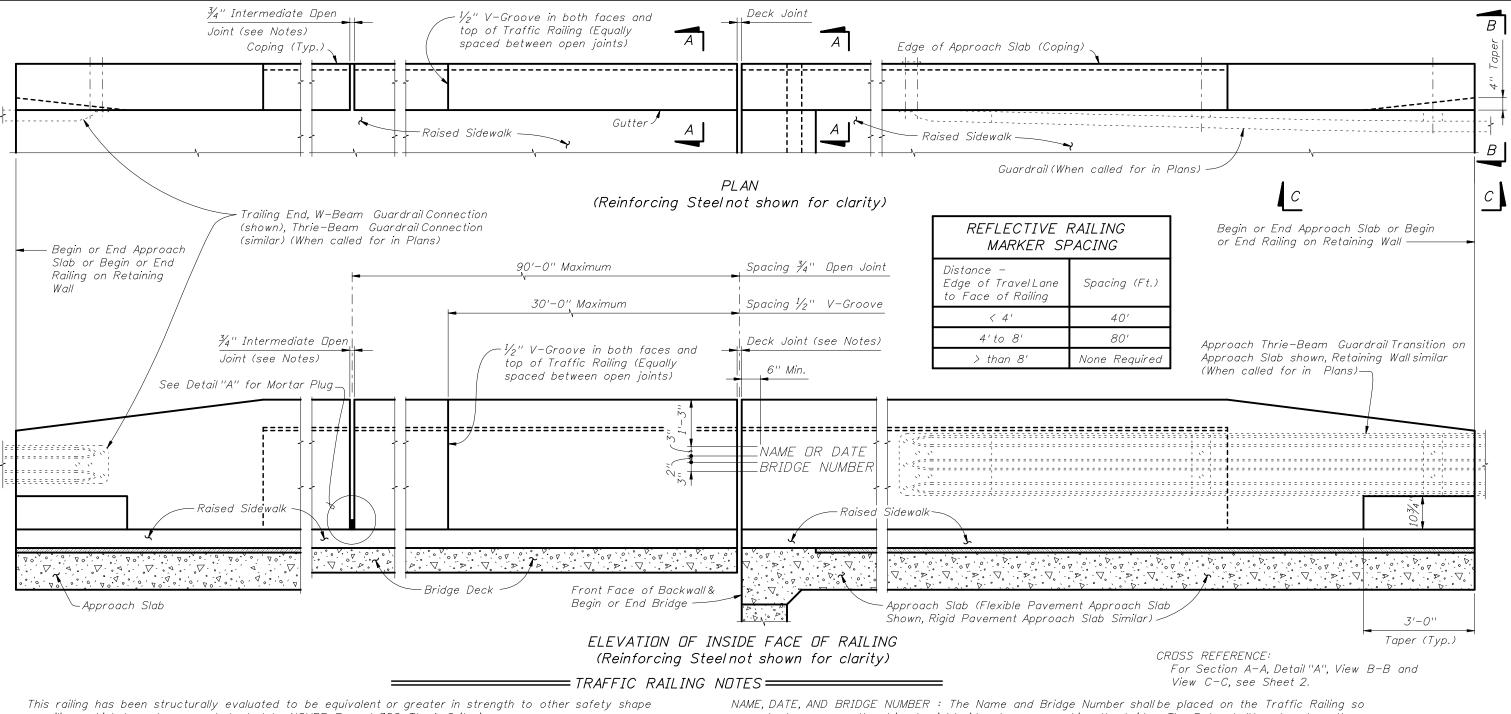
ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.120
Reinforcing Steel	LB/LF	23.29

(The above quantities are based on a crowned roadway, with a 2% cross slope)



Last Sheet No. 07/01/07 3 of 3

Index No. 421



railings which have been crash tested to NCHRP Report 350 TL-4 Criteria.

CONCRETE AND REINFORCING STEEL: See Structures Plans, General Notes.

MARKERS: Elevation Markers shall be placed on top of the Traffic Railing at the end bents. On bridges longer than 100 ft. one marker shall be placed at each end of the bridge. On bridges 100 ft. or less one marker shall be placed at one end of the bridge only. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The cost of installing the markers shall be included in the Contract Unit Price for the Traffic Railing.

GUARDRAIL: For Guardrail connection details, see Index No. 400.

RAILINGS ON RETAINING WALLS: If the Traffic Railing is to be provided on a retaining wall, the railing section will be the same as shown on Index No. 422, Sheet 2. All other details such as the guardrail transition attachment, the maximum spacing of the  $\frac{3}{4}$ " open joints and  $\frac{1}{2}$ " V-Groove shall apply.

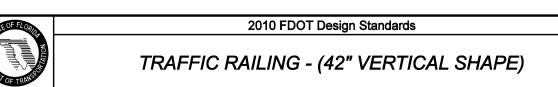
REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing.

V-GROOVES: Construct  $\frac{1}{2}$ " V-Grooves plumb and provide at 30'-0" maximum intervals as shown. Space V-Grooves equally between 3/4" Open Joints and/or Deck Joints and at V-Groove locations on Retaining

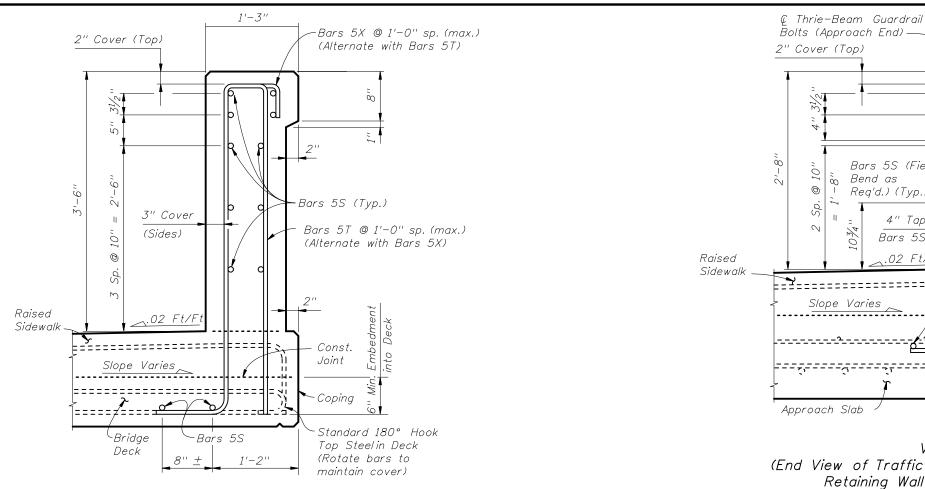
as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Name shall be as shown in the General Notes in the Structures Plans. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the year of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by  $\frac{3}{8}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures.

JOINTS: See Plans, Superstructure, Approach Slab and Retaining Walls Sheets for actual dimensions and joint orientation. Open Railing Joints at Deck Expansion Joint locations shall match the dimensions of the Deck Joint. For treatment of Railings on skewed bridges see Index No. 490. Deck Joint at Begin Bridge or End Bridge shown, Deck Joint at @ Pier or Intermediate Bent similar. Provide 3/4" Intermediate Open Joints at :

- (1) Substructure supports where superstructure slab is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.
- (4) At ends of approach slabs when adjacent to Retaining Walls and at expansion joints on Retaining Wall junction slabs.



Sheet No. 07/01/08 1 of 3



VIEW B-B (End View of Traffic Railing, Approach Slab shown, Retaining Wall Junction Slab similar)

Bars 5S (Field Bend as

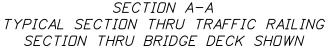
Req'd.) (Typ.)

1034'

4" Taper

Bars 5S

1'-3"



¢ W−Beam or Thrie-Beam Guardrail -Bolts (Trailing End)-Bolts (Approach End only) Bars 5X @ 1'-0" sp. (max.) Field Bend Bars 5T @ 1'-0" sp. (max.) Bars 5S as Required В Bars 5S Transition Bars 5T Field Cut, Lap Splice (2'-2'' min.)Transition Bars 5X Approach Slab Raised Sidewalk Field Cut & Lap 3'-0" Taper Splice (2'-2" min.)

6'-8"

CROSS REFERENCE: For location of Section A-A, Detail "A" and View B-B, see Sheet 1.

Transition Bars 5X (Field Cut and Lap

Rotate as Required to Maintain Cover)

3" Cover (Bars 5X)

5" Cover (Bars 5T)

-Const. Joint

Transition Bars 5T (Field Cut

and Lap Splice for Railing End

Transition, Shift and Rotate as

Required to Maintain Cover)

-Standard 180° Hook Top

Edge of Approach

Steelin Approach Slab

Slab (Coping)

Splice for Railing End Transition, Shift and

¢ W−Beam or Thrie-Beam Guardrail

Bolts (Trailing End)

# NOTES:

Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5T and 5X as required to maintain cover in Railing End Transition.

Omit Railing Taper, End Transition and Guardrail if Concrete Barrier Wall is used beyond the Approach Slab or Retaining Wall. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Taper and Railing End Transition is omitted, extend Typical Section to end of the Approach Slab or limiting station on Retaining Wall, and space Bars 5T and 5X at 1'-0' (Typ.)

VIEW C-C RAILING END TRANSITION (Guardrail Not Shown For Clarity)

# INSTRUCTIONS TO DESIGNER:

For Bridge Decks up to a maximum thickness of 9", the two Bars 55 placed in the Bridge Deck may substitute for the longitudinal deck steel located within the limits of Bars 5T, provided that the total area of longitudinal steel beneath the railing as required by calculation is not reduced. Show these bars on the Structures Plans, Superstructure Sheets with the deck steel.

All Bars 5S, 5T and 5X as shown are included in the Estimated Traffic Railing Quantities. Do not include Bars 5S, 5T and 5X in the reinforcing bar lists and estimated quantities for supporting bridge decks, approach slabs or retaining walls.



2010 FDOT Design Standards

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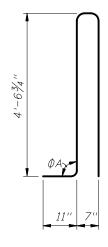
## CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL			
MARK	SIZE	LENGTH	
S	5	As Reqd.	
T	5	10'-8''	
Χ	5	6'-9''	

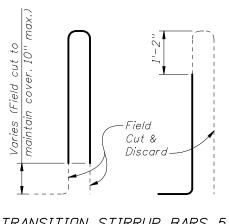
ROADWAY	$\phi_A$		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	83°	
6% to 10%	84°	96°	



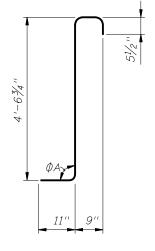
BAR 5S



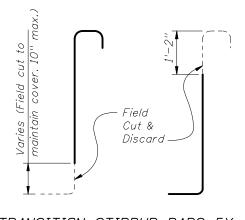
STIRRUP BAR 5T







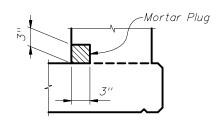
STIRRUP BAR 5X



TRANSITION STIRRUP BARS 5X To Be Field Cut (7 of each required per Railing End Transition)

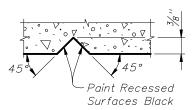
## REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The  $4'-6\sqrt[3]{4}''$  vertical dimension shown for Bars 5T and 5X is based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slope vary from the above amounts, adjust this dimension accordingly to achieve a 6" minimum embedment into the bridge deck. See Structures Plans, Superstructure and Approach Slab Sheets.
- 3. The reinforcement for the railing on a retaining wall shall be the same as detailed above with
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be
- 6. The Contractor may utilize Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

At Intermediate Open Joints, the lower 3" portion of the open joint shall be plugged by filling it with mortar in accordance with Section 400 of the Specifications.

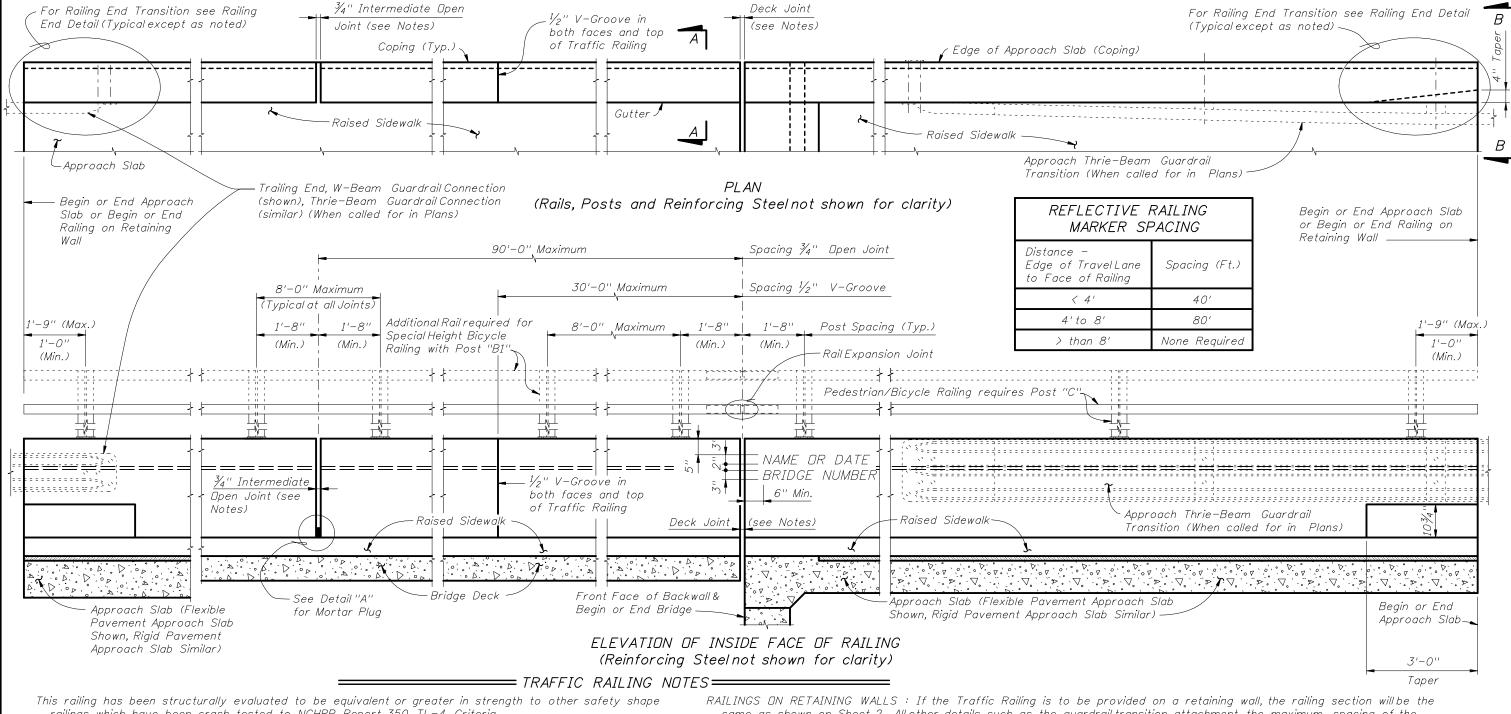


SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.145	
Reinforcing Steel	LB/LF	30.68	

(The above quantities are based on a 6'' thick x 6'wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope)





railings which have been crash tested to NCHRP Report 350 TL-4 Criteria.

CONCRETE AND REINFORCING STEEL: See Structures Plans, General Notes.

MARKERS: Elevation Markers shall be placed on top of the Traffic Railing at the end bents. On bridges longer than 100 ft. one marker shall be placed at each end of the bridge. On bridges 100 ft. or less one marker shall be placed at one end of the bridge only. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The cost of installing the markers shall be included in the Contract Unit Price for the Traffic Railing.

GUARDRAIL: For Guardrail connection details, see Index No. 400.

PEDESTRIAN/BICYCLE RAILING AND SPECIAL HEIGHT BICYCLE RAILING DETAILS : See Index No. 822 for Post, Rail and Rail Expansion Joint fabrication and installation Details and Notes.

 $V-GRODVES: Construct \frac{1}{2}"V-Grooves plumb. Space V-Grooves equally between \frac{3}{4}"$  Open Joints and/or Deck Joints and at V-Groove locations on Retaining Wall footings.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing.

same as shown on Sheet 2. All other details such as the quardrail transition attachment, the maximum spacing of the  $\frac{3}{4}$ " open joints and  $\frac{1}{2}$ " V-Groove shall apply.

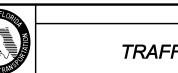
NAME, DATE, AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Name shall be as shown in the General Notes of the Structures Plans. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the year of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by  $rac{3}{8}$ " V–Grooves. V–Grooves shallbe formed by preformed letters and figures.

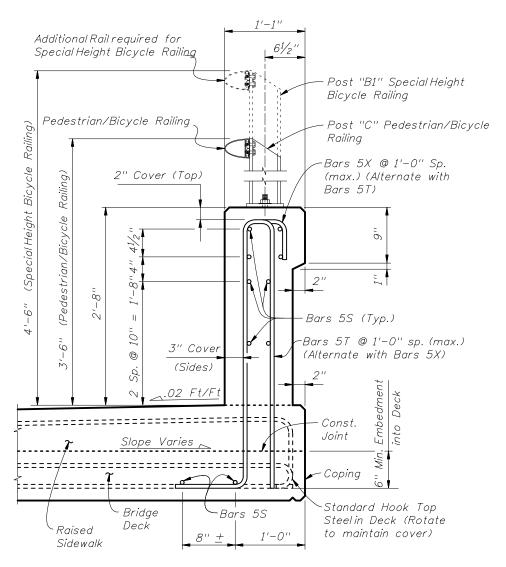
OPEN JOINTS : See Structures Plans, Superstructure, Approach Slab Sheets and Retaining Walls for actual dimensions and joint orientation. Open Railing Joints at Deck Expansion Joint locations shall match the dimensions of the Deck Joint. For treatment of Railings on skewed bridges see Index No. 490. Deck Joint at Begin or End Bridge Shown. Deck Joint at © Pier or Intermediate Bent Similar.

Provide  $\frac{3}{4}$ " Intermediate Open Joints at :

- (1) Substructure supports where superstructure slab is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.
- (4) At ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.

CROSS REFERENCE: For Section A-A and View B-B, see Sheet 2.

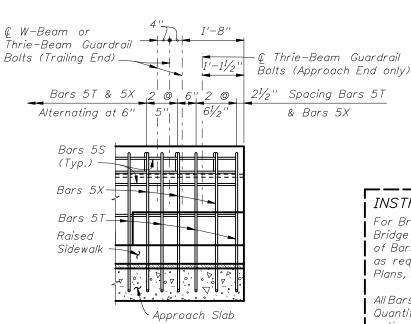




### SECTION A-A TYPICAL SECTION THRU TRAFFIC RAILING SECTION THRU BRIDGE DECK SHOWN

### NOTES:

Omit Railing End Taper and Guardrail if Concrete Barrier Wall is used beyond the Approach Slab. See Structures Plans, Plan and Elevation Sheet and Roadway Plans. If Railing End Taper is omitted, extend Typical Section to the end of the Approach Slab. Begin placing Railing Bars 5T and 5X on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of quardrail bolt holes. If required, adjustments to the bar spacing for Bars 5T and 5X shall be made immediately adjacent to Begin or End Bridge. Shift and rotate Bars 5T and 5X on Approach Slab in end taper section as required to maintain cover.



Additional Rail required for Special Height Bicycle Railing Post "B1" Special Height Bicycle Railing Pedestrian/Bicycle Railing . Post "C" Pedestrian/Bicycle -Bars 5X @ 1'-0" sp. (Special Height Bicycle 2" Cover (Top) (max.) (Alternate with Bars 5T) Thrie-Beam Guardrail Bolts (Trailing End) Guardrail Bolts (Approach End) -Bars 5S (Field Bend as Guardrail Bolts Required)(Typ.) -Bars 5T @ 1'-0'' sp. (max.) 0 4" Taper (Alternate with Bars 5X) \_.02 Ft/F Const. Joint γ Slope Varies \_ Standard Hook Top Bridge Bars 5S Steelin Deck (Rotate Deck Raised to maintain cover) 1'-0" Sidewalk Edge of Approach Slab (Coping) VIEW B-B

# APPROACH SLAB END VIEW OF TRAFFIC RAILING

CROSS REFERENCE: For location of Section A-A and View B-B see Sheet 1.

NOTE: For Post "B1", Post "C" and Rail Details, see Index No. 822.

### INSTRUCTIONS TO DESIGNER:

For Bridge Decks up to a maximum thickness of 9", the two Bars 5S placed in the Bridge Deck may substitute for the longitudinal deck steel located within the limits of Bars 5T, provided that the total area of longitudinal steel beneath the railing, as required by calculation, is not reduced. Show these bars on the Structures Plans, Superstructure Sheets with the deck steel.

All Bars 5S, 5T and 5X as shown are included in the Estimated Traffic Railing Quantities. Do not include Bars 5S, 5T and 5X in the reinforcing bar lists and estimated quantities for supporting bridge decks, approach slabs or retaining

RAILING END DETAIL



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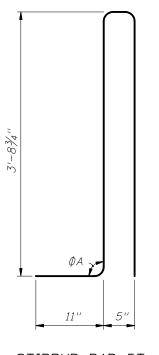
Sheet No. 01/01/08

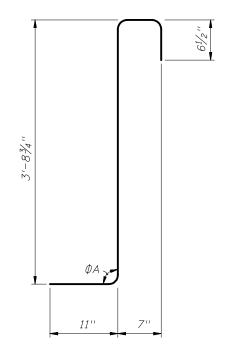
2 of 3

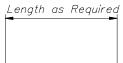
# CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL			
MARK	SIZE	LENGTH	
S	5	As Reqd.	
Τ	5	9'-0''	
X	5	5'-10''	

ROADWAY	ФА		
CROSS-SLOPE	LOW GUTTER	HIGH GUTTER	
0% to 2%	90°	90°	
2% to 6%	87°	93°	
6% to 10%	84°	96°	







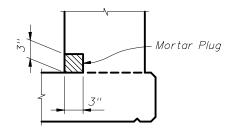
BAR 5S

STIRRUP BAR 5T

STIRRUP BAR 5X

### REINFORCING STEEL NOTES:

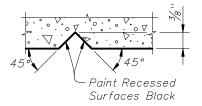
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The  $3'-8\frac{3}{4}''$  vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into
- 3. The reinforcement for the railing on a Retaining Wall shall be the same as detailed with  $\Phi A = 90^{\circ}$ .
- 4. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 5. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- 6. The Contractor may utilize Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



## DETAIL "A" - SECTION AT INTERMEDIATE OPEN JOINT

### NOTE:

At Intermediate Open Joints, the lower 3" portion of the open joint shall be plugged by filling it with mortar in accordance with Section 400 of the Specifications.

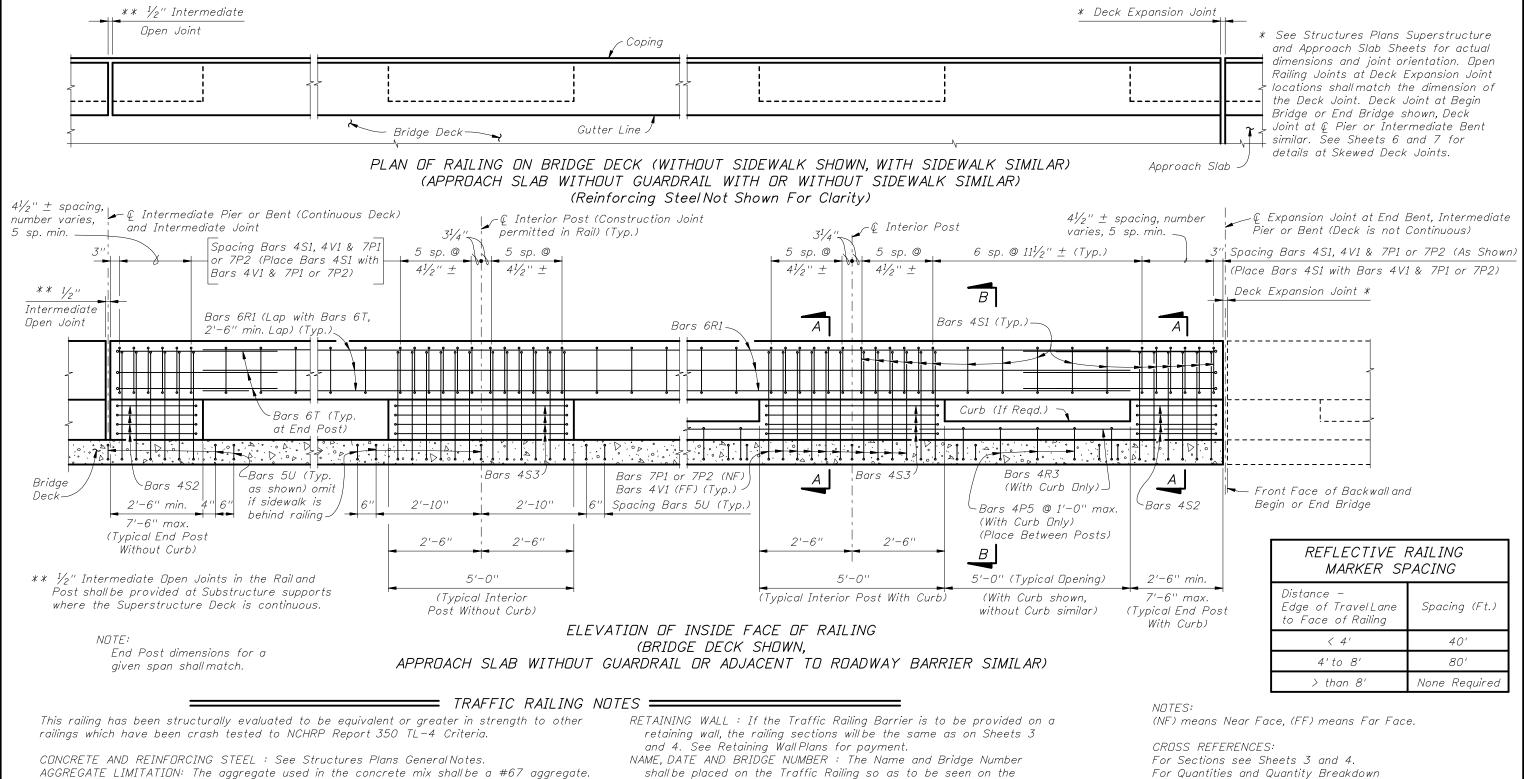


SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED TRAFFIC RAILING QUANTITIES			
ITEM	UNIT	QUANTITY	
Concrete	CY/LF	0.095	
Reinforcing Steel	LB/LF	25.90	

(The above quantities are based on a 6" thick x 6"wide raised sidewalk at low side of deck, 2% deck cross slope and counter 2% sidewalk cross slope.)





MARKERS: Elevation markers shall be placed on top of the Traffic Railing at the end bents. On bridges longer than 100 ft. one marker shall be placed at each end of the bridge. On bridges 100 ft. or less one marker shall be placed at one end of the bridge only. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The cost of installing the markers shall be included in the Contract Unit Price for the Traffic Railing.

GUARDRAIL: For Guardrail connection details see Index No. 400.

SUPERELEVATED BRIDGES: At the option of the Contractor the Traffic Railing on superelevated bridges may be constructed perpendicular to the roadway surface. The cost of all modifications will be at the Contractor's expense.

driver's right side when approaching the bridge. The Name shall be as shown in the General Notes in the Structures Plans. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the year of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by  $\frac{3}{8}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing.

see Sheet 5.

### INSTRUCTION TO DESIGNER

- 1. Indicate use of Curb beneath railing on low side of deck without sidewalks and other locations where required to contain bridge deck runoff. Define Curb location in Structures Plans Superstructure Sheets by Stationing limits or other appropriate methods.
- 2. Define lengths of End Posts in Structures Plans Superstructure Sheets.

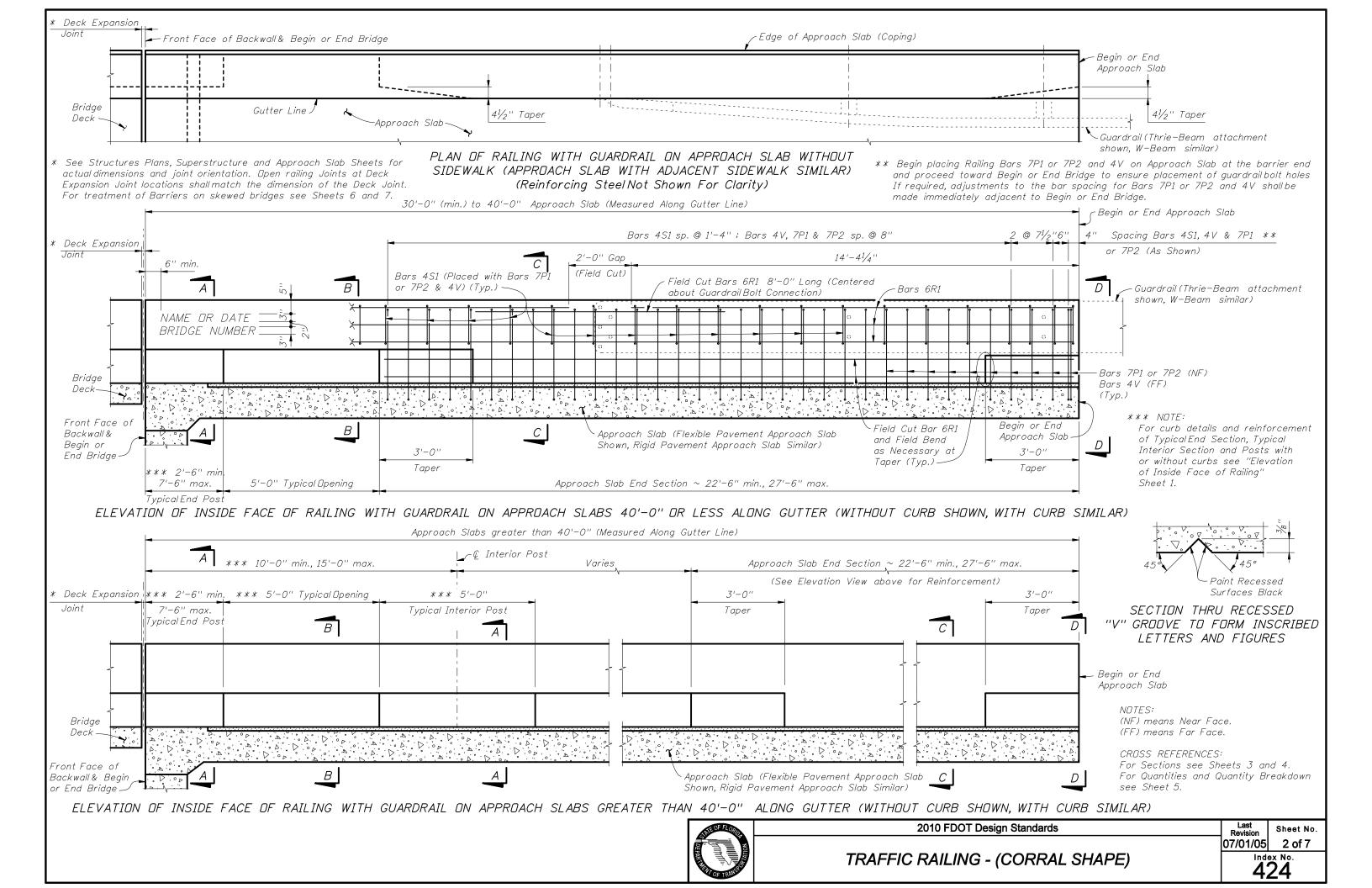


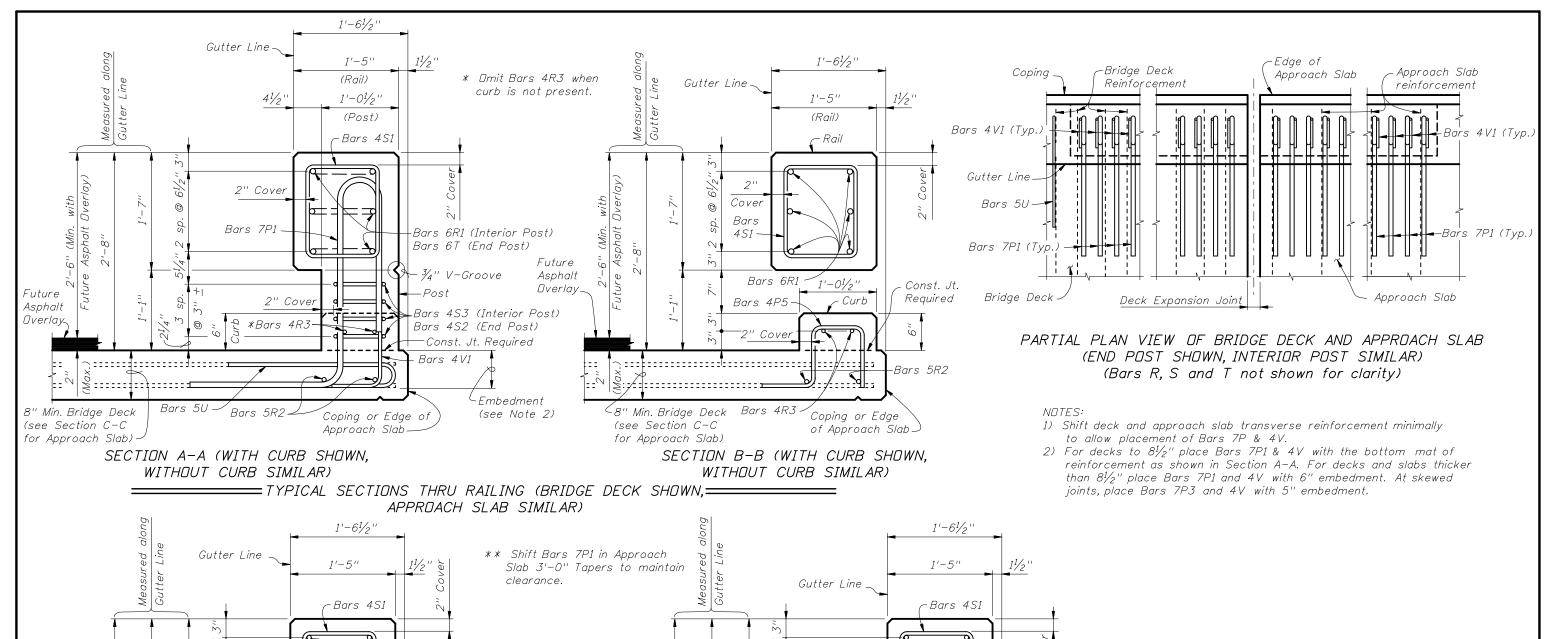
2010 FDOT Design Standards

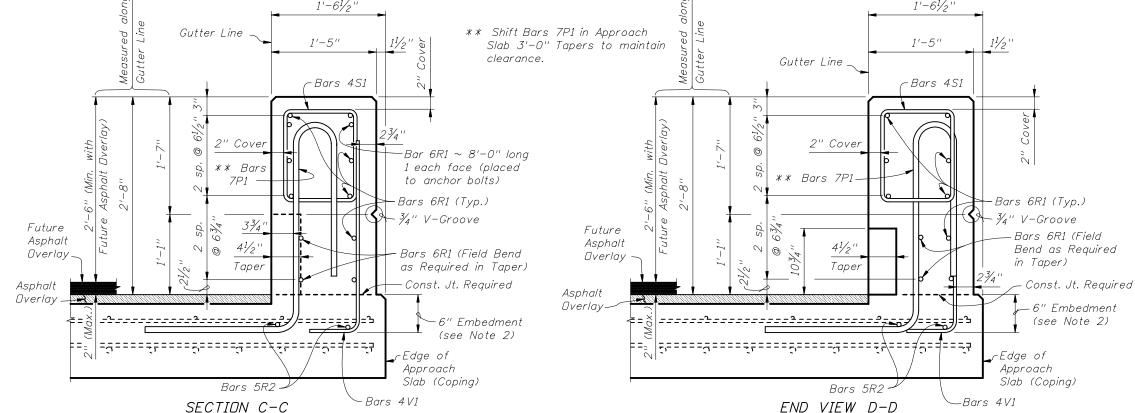
Sheet No. 07/01/08 1 of 7

TRAFFIC RAILING - (CORRAL SHAPE)

1ndex No. 424







CROSS REFERENCES: For Locations of Sections see Sheets 1 and 2. For Quantities and Rebar Details see Sheet 5.

\_\_\_\_\_\_TYPICAL SECTIONS THRU RAILING END SECTIONS ON APPROACH SLAB WITH GUARDRAIL \_\_\_\_\_\_ (APPROACH SLAB (FLEXIBLE PAVEMENT APPROACHES) SHOWN, APPROACH SLAB (RIGID PAVEME<u>NT APPROACHES) SIMILAR)</u>

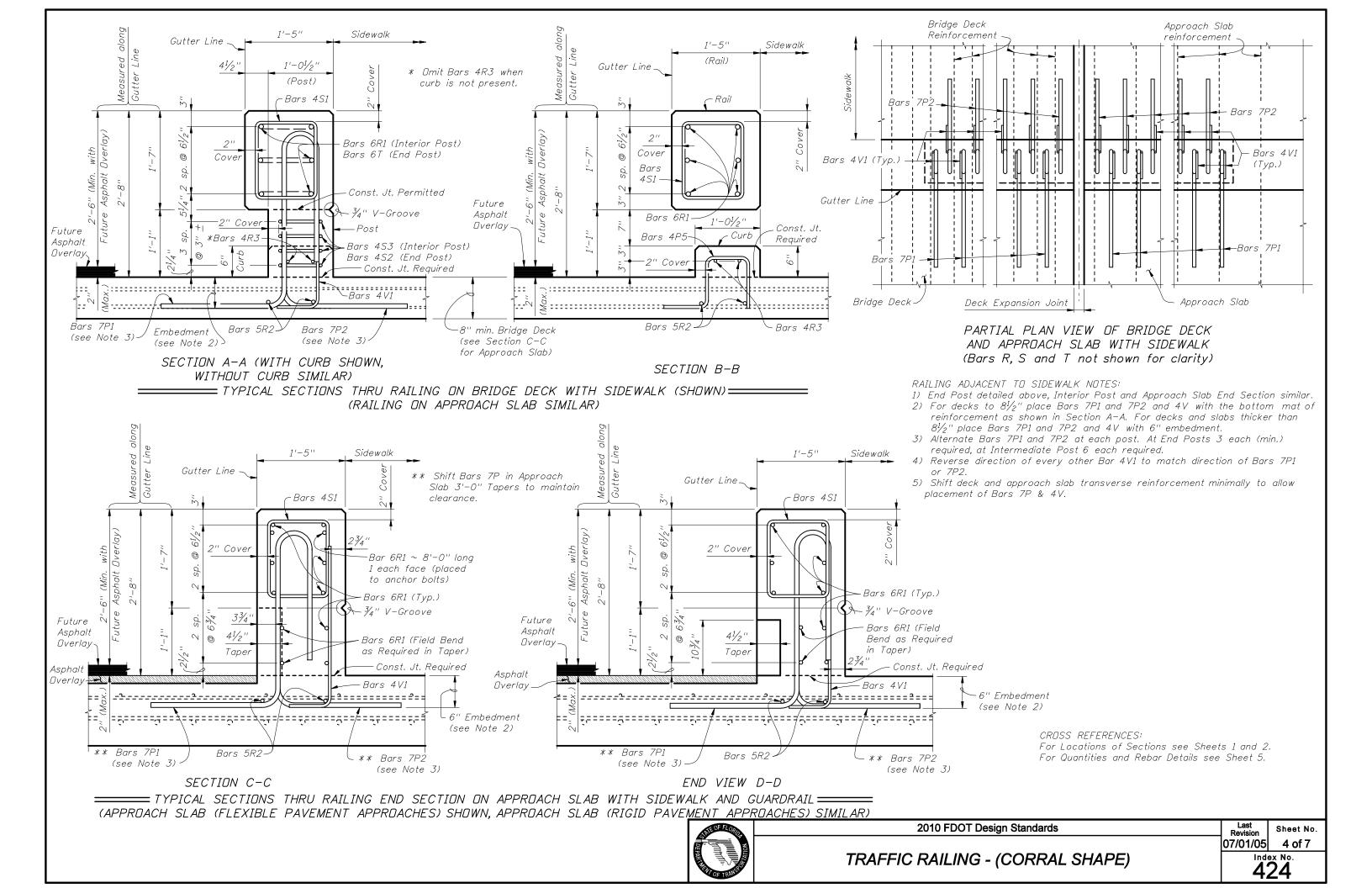


2010 FDOT Design Standards

TRAFFIC RAILING - (CORRAL SHAPE)

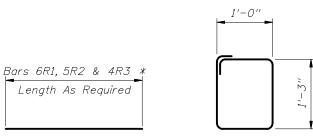
Last Revision 07/01/05 3 of 7

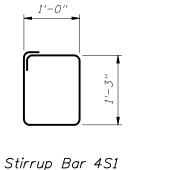
Index No. 424

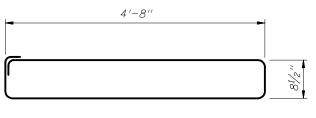


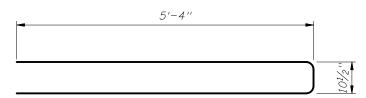
### BILL OF REINFORCING STEEL SIZE LENGTH LB/BAR MARK 7'-4" 15.00 P1 7 P2 7'-3" 14.82 P3 7'-2" 14.65 \*\*\* P4 7'-3" 14.82 \* P5 4 2'-11'' 1.94 1.5 (LB/LF) R1 As Regd. R2 5 As Regd. 1.04 (LB/LF) \* R3 As Regd. 0.67 (LB/LF \*\* S1 5'-0" 3.34 Varies Varies \*\* S2 6'-3" min 4.18 min. '6'-3" max 10.86 max. \*\* S3 11'-3'' 7.52 T 11'-4'' 17.02 4.87 4'-8" U V13'-2" 2.12 \*\*\* V2 3'-6" 2.34

- \* Bars 4P5 and 4R3 are to be used with a curb only.
- \*\* Bend Bars 4S1, 4S2 & 4S3 around a #3 Stirrup Pin.
- \*\*\* Bars 7P4 & 4V2 are to be used on CIP Concrete Retaining Walls.









Stirrup Bar 4S3

ΦА

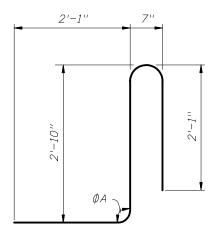
Bar 7P3 (Requires

Stirrup Bar 6T

23/4" Ø Pin

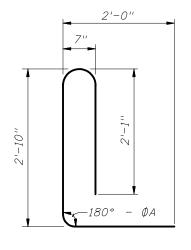
Stirrup Bar 4S2

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

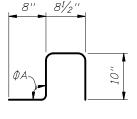


Bar 7P1

Bars 6R1, 5R2 & 4R3



Bar 7P2



Bar 4P5 \* Stirrup Bar 5U

ine Perpendicular or Radial to Gutter

Top of CIP Concrete Retaining Wall

Bar 4V1

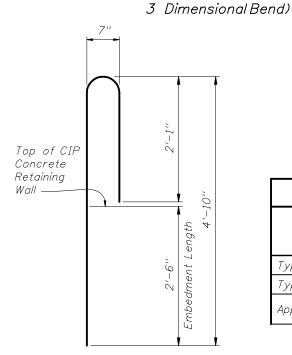
Bar 4V2 \*\*\*

### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a CIP Concrete Retaining Wall shall be the same as detailed above for a 8" deck with  $\phi A = 90^{\circ}$ , where applicable. If bottom horizontallegs of Bars 7P1, 7P3 and 4V1 prohibit placement, Bars 7P4 and 4V2 may be substituted for Bars 7P1, 7P3 and 4V1 as shown.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover unless otherwise noted.
- 4. At Construction Joints Bars 6R1, 5R2 and 4R3 may be continuous or spliced. Where bars are spliced provide a 2'-6" min. lap length for Bar 6R1, a 2'-0" min. lap length for Bars 5R2 and a 1'-3" min. lap length for Bars 4R3.
- 5. The skew angle for Bars 7P3 may vary from joint to joint and side to side, see Structures Plans, Superstructure Sheets for details.

RDADWAY DR SIDFWALK	HIGH SIDE	LOW SIDE
JIDL WALK	SIDE	SIDE
CROSS-SLOPE	ФА	ΦA
0% to 2%	90°	90°
2% to 6%	93°	87°
6% to 10%	96°	84°

*\$\PhiA\$* shall be 90° if Contractor elects to place Railing Perpendicular to the Deck.



Parallel to Joint

Skew Angle (measured in

the horizontal plane, see

Superstructure Sheets)

Structures Plans,

ESTIMATED TRAFFIC RAILING QUANTITIES		
ITEM	CONCRETE QUANTITY (CY)	REBAR QUANTITY (LB)
Typical10'-0" Section w/Curb	1.13	451
Typical10'-0" Section w/o Curb	1.03	428
Approach Slab with GuardrailEnd Section	0.14 (per LF)	44 (per LF)

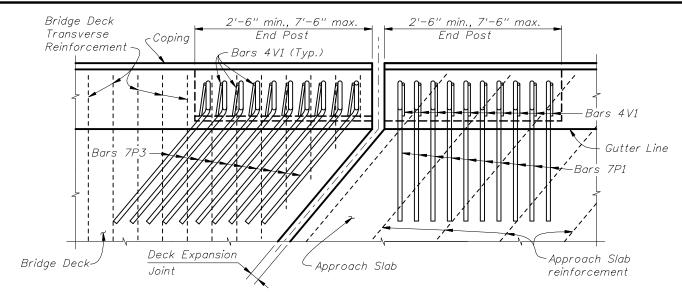
Bar 7P4 \*\*\*



Sheet No. 07/01/05 5 of 7

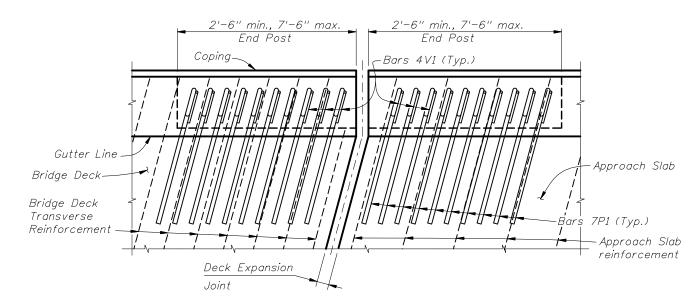
TRAFFIC RAILING - (CORRAL SHAPE)

1ndex No. 424

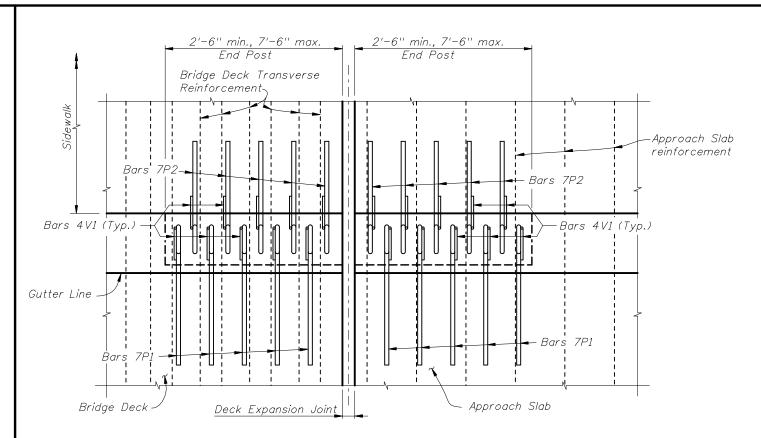


PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB - SKEW ANGLE GREATER THAN 15 DEGREES

- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the autter line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):
- 3) End Post & Approach Slab End Section Place Bars 7P1 & 4V1 in obtuse corners of intersection of deck joint and gutter line. Place Bars 7P3 & 4V1 in acute corners of intersection of deck joint and gutter line as required. Interior Post - use Bars 7P1 and 4V1 placed with bottom mat of reinforcement. Shift deck or slab reinforcement minimally to allow proper placement of Bars 7P and 4V and to facilitate placement of concrete. APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 4) Place Bars 7P1 & 4V1 in obtuse corners of intersection of deck joint and gutter line and Bars 7P3 & 4V1 in acute corners of intersection of deck joint and gutter line as required. Shift deck or slab reinforcement minimally to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete.
- 5) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of quardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB - SKEW ANGLE 15 DEGREES OR LESS



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK - O DEGREE SKEW ANGLE

- 1) Alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 as detailed above to facilitate placement of concrete.
- 2) Shift deck transverse reinforcement minimally to allow placement of Bars 7P & 4V.

- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- 3) Bars 7P & 4V in the Approach Slab may be rotated to match Approach Slab reinforcement or placed perpendicular or radial to the gutter line.
- BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):
- 4) Rotate vertical Bars 7P & 4V to match bridge deck reinforcement. Shift deck & slab transverse reinforcement to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete. APPROACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 5) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.
- 6) Bars 7P at end of the railing shall be field cut and shifted to maintain clearance, see Railing End Taper Detail Sheet 2 for similar details.

1) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar. 2) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets

for skew angles, joint orientation, dimensions and details.

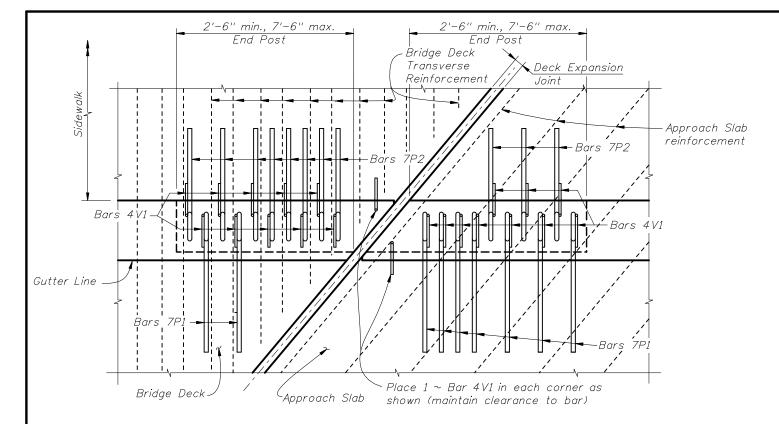


2010 FDOT Design Standards

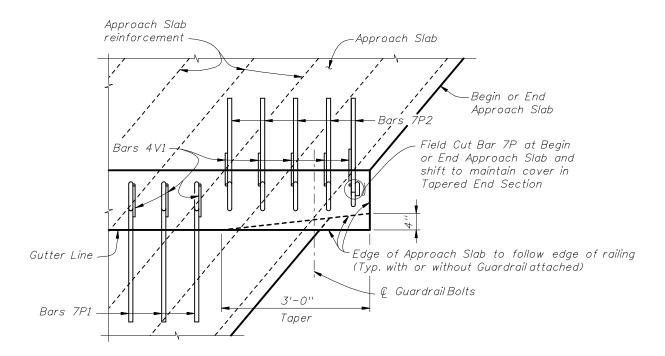
Sheet No. 07/01/05 6 of 7

TRAFFIC RAILING - (CORRAL SHAPE)

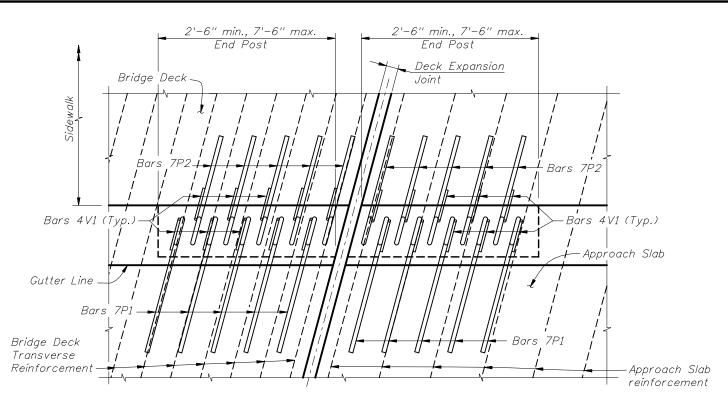
1ndex No. 424



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK
- SKEW ANGLE GREATER THAN 15 DEGREES



PARTIAL PLAN VIEW AT BEGIN OR END APPROACH SLAB WITH SIDEWALK AND RAILING WITH GUARDRAIL ATTACHED - SKEW ANGLE GREATER THAN 15 DEGREES SHOWN, 15 DEGREES OR LESS SIMILAR



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK
- SKEW ANGLE 15 DEGREES OR LESS

### VOTES:

- 1) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.
- 3) Edge of Approach Slab adjacent to the roadway shall follow end of railing, Bars 7P at end of the railing shall be field cut and shifted to maintain clearance, see detail bottom left this sheet for similar details.

  BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (SHOWN):
- 4) Alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 to facilitate placement of concrete.
- 5) Bars 7P & 4V shall be rotated to match bridge deck reinforcement. Shift deck transverse reinforcement minimally to allow placement of Bars 7P & 4V.
- 6) Railing End Post and reinforcement detailed above. Railing Interior Post reinforcement similar. APPRDACH SLAB WITH GUARDRAIL ATTACHED (NOT SHOWN):
- 7) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.

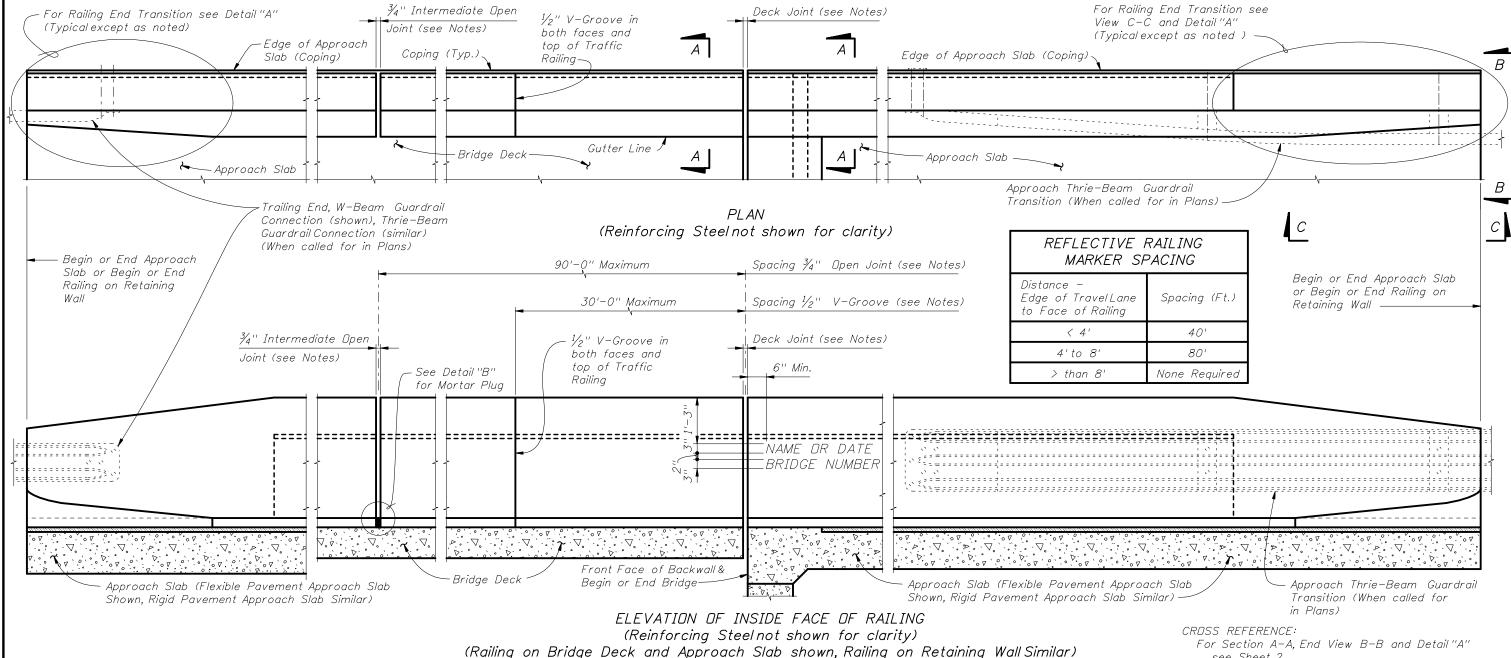
### NOTES

- 1) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 2) Bars 4S1 (not shown) shall be placed perpendicular or radial to the gutter.

BRIDGE DECK AND APPROACH SLAB WITHOUT GUARDRAIL ATTACHED (NOT SHOWN):

- 3) Deck transverse reinforcement may be shifted minimally as required to allow proper placement of Bars 7P & 4V and to facilitate placement of concrete. Bars 7P1 & 4V1 or 7P2 & 4V1 shall be used on opposing sides of the joint depending on the direction of the skew, see Detail above. Approach Slab reinforcement may be shifted if conflicts occur.
- 4) Interior Post alternate Bars 7P1 with Bars 7P2 and reverse direction of every other Bar 4V1 to facilitate placement of concrete.
- 5) End Post alternate Bars 7P1 with Bars 7P2 and reverse direction of Bars 4V1 (as detailed) where possible. APPROACH SLAB WITH GUARDRAIL ATTACHED (SHOWN):
- 6) Use Bars 7P2 and reverse direction of Bars 4V1 where skew restricts use of Bars 7P1 & 4P1.
- 7) Begin placing Railing Bars 7P & 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 7P & 4V shall be made immediately adjacent to Begin or End Bridge.





= TRAFFIC RAILING NOTES =

see Sheet 2. For Detail "B" see Sheet 3.

This railing has been structurally evaluated to be equivalent or greater in strength to other safety shape railings which have been crash tested to NCHRP Report 350 TL-5 Criteria.

CONCRETE AND REINFORCING STEEL: See Structures Plans, General Notes.

MARKERS: Elevation Markers shall be placed on top of the Traffic Railing Railing at the end bents. On bridges longer than 100 ft. one marker shall be placed at each end of the bridge. On bridges 100 ft. or less one marker shall be placed at one end of the bridge only. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor. The cost of installing the markers shall be included in the Contract Unit Price for the Traffic Railing.

SUPERELEVATED BRIDGES: At the option of the Contractor the Traffic Railing on superelevated bridges may be constructed perpendicular to the roadway surface. If an adjoining railing is constructed plumb, transition the end of the Traffic Railing from perpendicular to plumb over a minimum distance of 20'-0". The cost of all modifications will be at the Contractor's expense.

GUARDRAIL: For Guardrail connection details, see Index No. 400.

RAILINGS ON RETAINING WALLS: If the Traffic Railing is to be provided on a retaining wall, the railing section will be the same as shown on Sheet 2. All other details such as the quardrail transition attachment, the maximum spacing of the  $\frac{3}{4}$ " open joints and  $\frac{1}{2}$ " V-groove shall apply.

V-GRODVES: Construct  $\frac{1}{2}$ " V-Grooves plumb, Space V-Grooves equally between  $\frac{3}{4}$ " Open Joints and/or Deck Joints and at V-Groove locations on Retaining Wall footings.

NAME, DATE, AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Name shall be as shown in the General Notes in the Structures Plans. The Date shall be the year the bridge is completed. For a widening when the existing railing is removed, use both the existing date and the vear of the widening. Black plastic letters and figures 3" in height may be used, as approved by the Engineer, in lieu of the letters and figures formed by  $\frac{3}{8}$ " V-Grooves. V-Grooves shall be formed by preformed letters and figures. JOINTS: See Structures Plans, Superstructure, Approach Slab and Retaining Walls Sheets for actual dimensions and joint orientation. Open Railing Joints at Deck Expansion Joint locations shall match the dimensions of the Deck Joint. For treatment of Railings on skewed bridges see Index No. 490. Deck Joint at Begin or End Bridge Shown. Deck Joint at @ Pier or Intermediate Bent Similar.

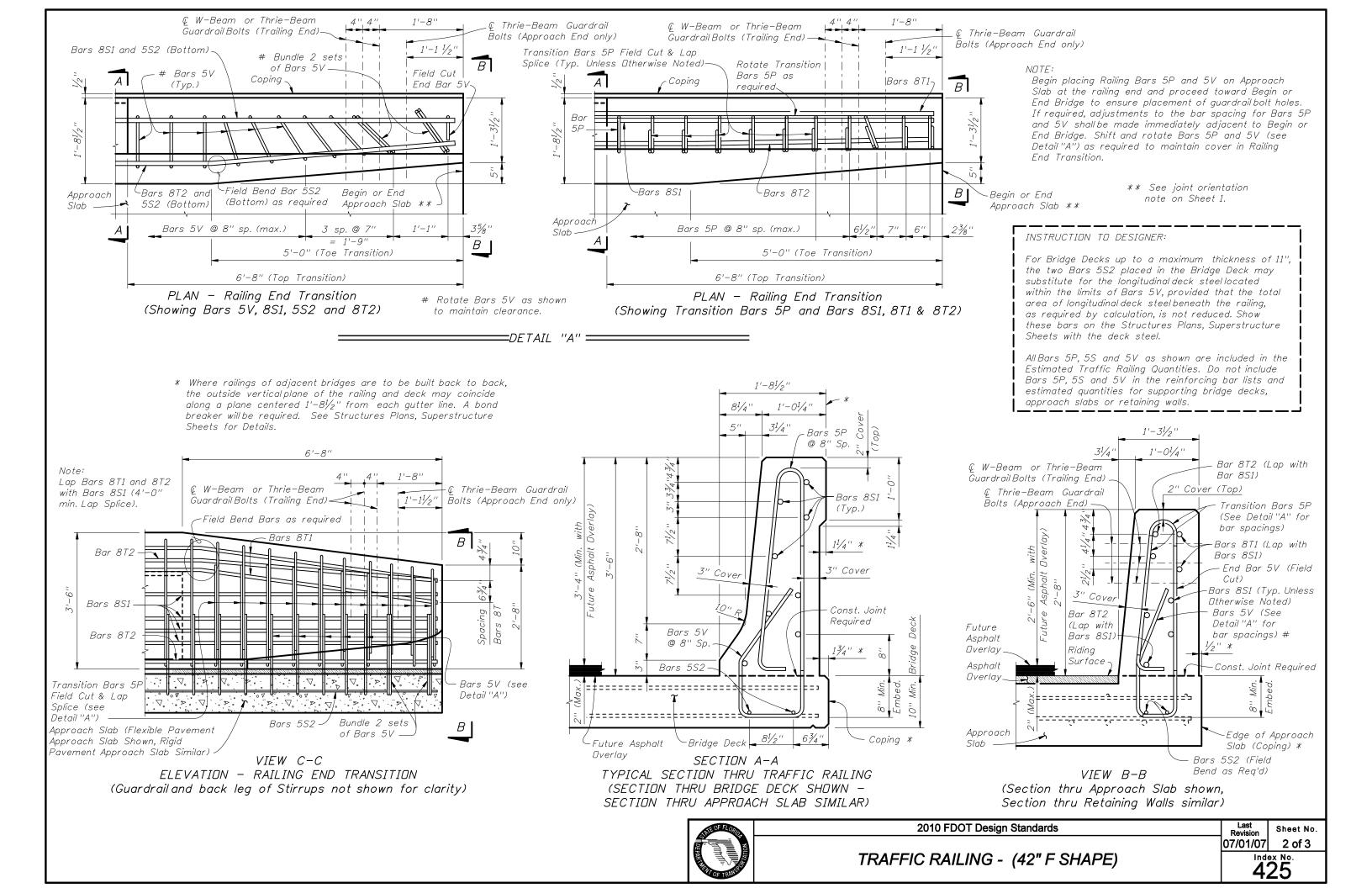
Provide  $\frac{3}{4}$ " Intermediate Open Joints shall be provided at :

- (1) Substructure supports where superstructure slab is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.
- (4) At ends of approach slabs when adjacent to retaining walls and at expansion joints on retaining wall junction slabs.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table above. Reflector color (white or yellow) shall match the color of the near edgeline. The cost of the reflective markers shall be included in the Contract Unit Price for the Traffic Railing



Sheet No. 07/01/08 1 of 3

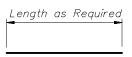


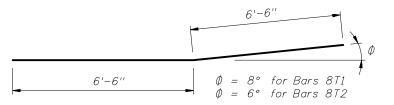
### CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS

BILL OF REINFORCING STEEL		
MARK	SIZE	LENGTH
Р	5	7'-5''
S1	8	As Reqd.
<i>S2</i>	5	As Reqd.
T1 & T2	8	13'-0''
V	5	6'-2"

ROADWAY	LOW C	GUTTER	HIGH (	GUTTER
CROSS-SLOPE	ФА	ΦB	ФА	ΦВ
0% to 2%	90°	90°	90°	90°
2% to 6%	93°	87°	87°	93°
6% to 10%	96°	84°	84°	96°

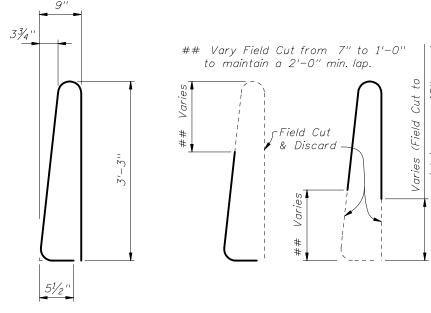
 $\phi A$  and  $\phi B$  shall be 90° if Contractor elects to place Railing perpendicular to the Deck.

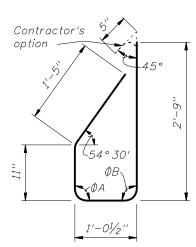


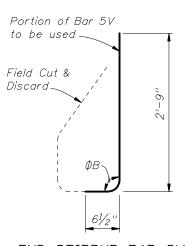


TRANSITION BARS 8T1 & 8T2 (2 of each required per Railing End Transition)

BARS 8S1 & 5S2







STIRRUP BAR 5P

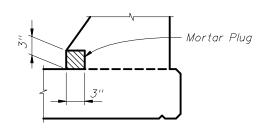
TRANSITION STIRRUP BARS 5P To Be Field Cut (10 of each required per Railing End Transition)

STIRRUP BAR 5V

END STIRRUP BAR 5V To Be Field Cut (One required per Railing End Transition)

### REINFORCING STEEL NOTES:

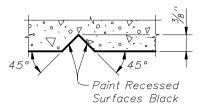
- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the railing on a retaining wall shall be the same as detailed above for a 10" deck with  $\phi A = \phi B = 90^{\circ}$ .
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 8S1 may be continuous or spliced at the construction joints. Lap splices for Bars 8S1 and 5S2 shall be a minimum of 4'-0" and 2'-0", respectively.
- 5. The Contractor may utilize Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement shall conform to ASTM A497.



# DETAIL "B" - SECTION AT INTERMEDIATE OPEN JOINT

NOTE:

At Intermediate Open Joints, the lower 3" portion of the open joint shall be plugged by filling it with mortar in accordance with Section 400 of the Specifications.

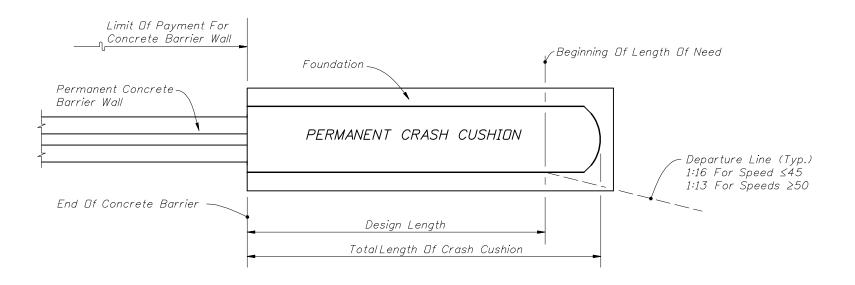


SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED LETTERS AND FIGURES

ESTIMATED T	TRAFFIC RAILING	QUANTITIES
ITEM	UNIT	QUANTITY
Concrete	CY/LF	0.154
Reinforcing Steel	LB/LF	44.71

The estimated railing quantities are based on a 2% deck cross slope; railing on low side of deck.





### DESIGN NOTES - CONCRETE BARRIER WALL APPLICATION

- 1. Design length is the length from the beginning of length of need to the end of the crash cushion.
- 2. Determine length of need for barrier as detailed on Index 400.
- 3. Establish the end of barrier based on design length of shortest Crash Cushion option for given design speed.
- 4. Determine that adequate space is available for construction of all options for given design speed. If adequate space is not available, options must be limited to those that will fit. Tabulate selected options in the plans by location and design speed.

### GENERAL NOTES FOR OPTIONAL CRASH CUSHIONS

- 1. Crash Cushions for which the optionalitem may be used are limited to the systems identified on this index. The Contractor may only use the options identified in the plans.
- 2. This Index is applicable for permanent installations that shield the ends of Concrete Barrier Walls or Guardrails only.
- 3. For Crash Cushion details, see drawings posted on Qualified Products List (QPL) web page.
- 4. For other Crash Cushion applications, see the approved QPL drawings.
- 5. Crash Cushions shall be assembled and installed in accordance with the manufacturer's specifications and any limiting conditions noted on the approved QPL drawings.
- 6. Transition Panels may be required from Concrete Barriers to Crash Cushions subject to reverse direction hits; see the Crash Cushion drawings posted on the QPL for details. Transitions are required between the Crash Cushion and guardrail and vary in length depending on the Crash Cushion used; see the Crash Cushion drawings for details.

The cost of the transition(s) is to be included in the cost of the Crash Cushion.

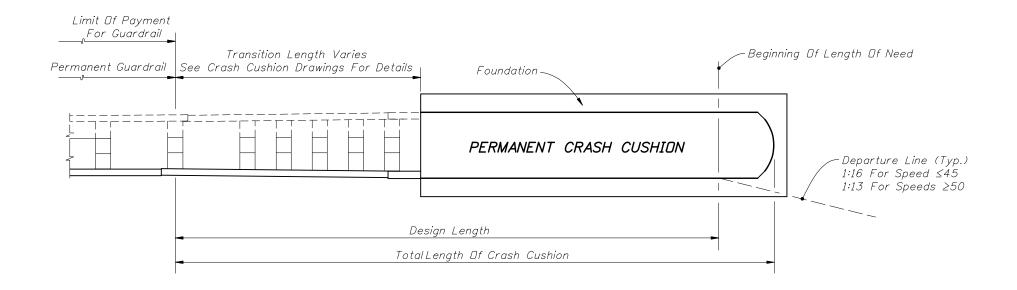
7. Optional Crash Cushions will be paid for under the contract unit price for Vehicular Impact Attenuator/Crash Cushion (optional) EA, and shall be full compensation for furnishing and installing all components in accordance with the plans; the manufacturer's detail drawings, procedures and specifications; the drawings posted on the QPL and this Index.

Design Speed	System	Design Length (Ft.)	TotalLength Of Crash Cushion (Ft.)
30	QuadGuard	6.71	9.98
	TAU 11	4.61	8.35
	SHORTRACC	14.11	15.06
	QuadGuard	6.71	9.98
<i>35</i>	TAU 11	7.45	11.19
	SHORTRACC	14.11	15.06
40	QuadGuard	6.71	9.98
	TAU 11	10.30	14.04
	SHORTRACC	14.11	15.06
	QuadGuard	9.55	12.83
45	TAU 11	10.30	14.04
	SHORTRACC	14.11	15.06
	QuadGuard	12.55	15.83
50	TAU 11	13.14	16.88
	TRACC	21.00	21.98
	QuadGuard	15.65	18.93
55	TAU 11	18.82	22.56
	TRACC	21.00	21.98
	QuadGuard	18.62	21.90
60	TAU 11	21.67	25.41
	TRACC	21.00	21.98
	QuadGuard	21.60	24.87
C.F.	QuadGuard HS	24.58	29.16
65	TAU 11	24.52	28.26
	FASTRACC	26.00	26.98
	QuadGuard	27.55	30.83
70	QuadGuard HS	24.58	29.16
70	TAU 11	27.36	31.10
	FASTRACC	26.00	26.98

NOTE:

Total length of Crash Cushion for the TAU II units is based on use of the Compact Backstop. When the PCB Backstop is used, these lengths are reduced by 1.67 ft.





### DESIGN NOTES - GUARDRAIL APPLICATION

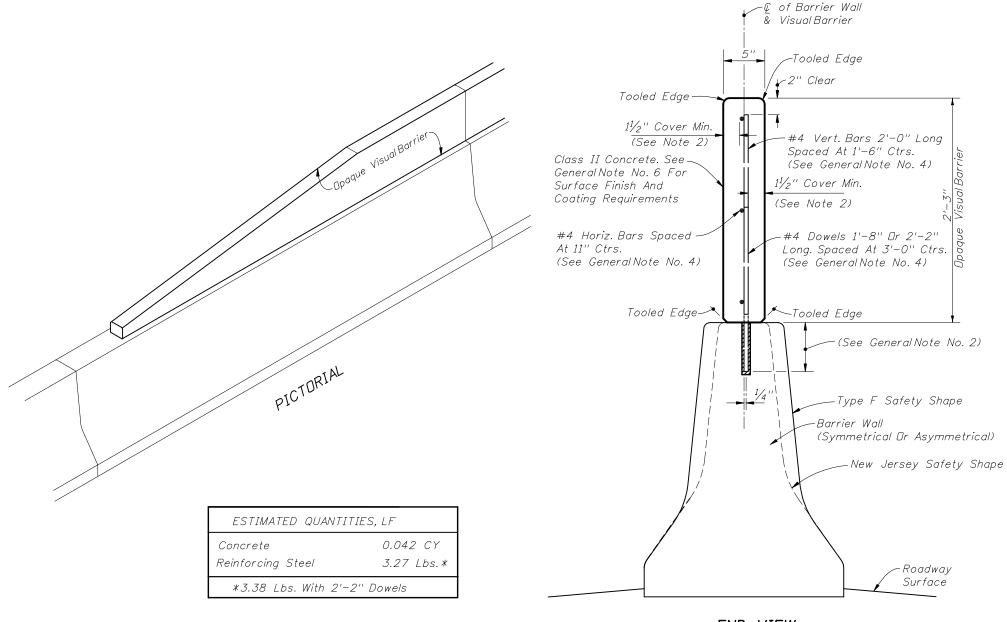
- 1. Design length is the length from the beginning of length of need to the end of the transition section.
- 2. Determine length of need for barrier as detailed on Index 400.
- 3. Establish the end of the guardrail based on design length of shortest Crash Cushion option for given design speed.
- 4. Determine that adequate space is available for construction of all options for given design speed. If adequate space is not available, options must be limited to those that will fit. Tabulate selected options in the plans by location and design speed.

GUARDRAIL APPLICATION				
Design Speed	System	Design Length (Ft.)	TotalLength Of Crash Cushion (Ft.)	
	QuadGuard	26.29	29.56	
30	TAU 11	18.36	22.10	
	SHORTRACC	26.70	27.65	
	QuadGuard	26.29	29.56	
35	TAU 11	21.20	24.94	
	SHORTRACC	26.70	27.65	
	QuadGuard	26.29	29.56	
40	TAU 11	24.05	27.79	
	SHORTRACC	26.70	27.65	
	QuadGuard	29.13	32.41	
45	TAU 11	24.05	27.79	
	SHORTRACC	26.70	27.65	
	QuadGuard	32.13	35.41	
50	TAU 11	26.89	30.63	
	TRACC	33.59	34.57	
	QuadGuard	35.23	38.51	
55	TAU 11	32.57	36.31	
	TRACC	33.59	34.57	
	QuadGuard	38.20	41.48	
60	TAU 11	35.42	39.16	
	TRACC	33.59	34.57	
	QuadGuard	41.18	44.45	
	QuadGuard HS	44.16	48.74	
65	TAU 11	38.27	42.01	
	FASTRACC	38.59	39.57	
	QuadGuard	47.13	50.41	
	QuadGuard HS	44.16	48.74	
70	TAU 11	41.11	44.85	
	FASTRACC	38.59	39.57	



# End Measurement For Opaque Visual Barrier Payment, LF 6' Transition Cut & Field Bend Reinf. Steel 2" Clear 2" Clear Top Df Concrete Barrier Wall

### ELEVATION OF REINFORCEMENT AND DOWELING



END VIEW

### GENERAL NOTES

- 1. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when yielding to such strikes.
- 2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be 1'-8" in length, embedded 6" into the barrier wall and set with an approved chemical grout. Embedment holes shall be \(^{5}\%''\) diameter, drilled to a depth \(^{1}\/\_4\)" below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

When longitudinal reinforcing bars are encountered in the stem of existing barrier, shift the dowels to clear, maintaining the  $1\frac{1}{2}$ " Cover Minimum to the face of the Opaque Visual Barrier.

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structures, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in the plans.

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

- 4. In lieu of the reinforcement shown, the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer. Details shall be submitted with requests for substitution.
- 5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineer's approval.

The Contractor may construct the opaque screen monolithically with the barrier wall; however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations in Note No. 1 above.

- 6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 521 of the Standard Specification, unless another finish is called for in the plans. The surfaces shall have a Class 5 Applied Finish Coating in accordance with Section 400 only when called for in the plans.
- 7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height), LF.



1ndex No.

### ===== TRAFFIC RAILING NOTES ======

This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested in accordance with NCHRP Report 350 TL-4 criteria.

CONCRETE: Concrete for Transition Blocks and Curbs shall be Class II (Bridge Deck).

REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60.

THRIE-BEAM GUARDRAIL: Steel Thrie-Beam Elements shall meet the requirements for Class B (10 Gauge) Guardrail of AASHTO M 180, Type II (Zinc coated). The minimum panel length for Thrie-Beam Elements shall be 12'-6''. Field drilled holes for Post connections shall be  $\frac{3}{4}$ '' by  $2\frac{1}{2}$ '' slotted holes.

GUARDRAIL BOLTS: Guardrail bolts, nuts and washers shall be in accordance with AASHTO M180.

GUARDRAIL POSTS AND BASE PLATES: Posts and Base Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

ANCHOR BOLTS, NUTS AND WASHERS: Adhesive—Bonded Anchors and Anchor Bolts shall be fully threaded rods in accordance with ASTM F1554 Grade 105 or ASTM A193 Grade B7. At the Contractor's option, Anchor Bolts for through bolting may be in accordance with ASTM 449. All Nuts shall be single self—locking hex nuts and in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only) shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and the exposed trimmed ends of anchors shall be coated with a galvanizing compound in accordance with the Specifications.

CDATINGS: All Nuts, Bolts, Anchors, Washers, Guardrail Posts, Anchor Plates and Base Plates shall be hot-dip galvanized in accordance with the Specifications. Guardrail Post Assemblies shall be hot-dip galvanized after fabrication.

ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 15,000 lbs. for  $\frac{7}{8}$ "  $\phi$  anchor bolts; 55,000 lbs. for the  $\frac{1}{4}$ " anchor bolts with 13" embedment; and 30,500 lbs. for the  $\frac{1}{4}$ "  $\phi$  anchor bolts with 5" embedment.

BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments. Details for bridges on horizontally curved alignments are similar.

PDST SPACING: Posts shall be located along the length of the bridge at typical 6'-3" or 3'-1 ½" spaces. Utilize the Modified Post Spacing at Intermediate Deck Joints Details as required to clear deck joints. Establish post spacing along the bridge and Roadway Guardrail Transition beginning with the Key Post. The variable post spacings located near begin and end bridge may be utilized to optimize the typical post spacing. Variable lengths of guardrail overlap are also permitted to optimize the typical post spacing. Symmetry of post spacing is not necessary.

THRIE-BEAM EXPANSION SECTION: Thrie-Beam Expansion Sections shall be installed at locations shown in the Plans. Install nuts for splice bolts finger-tight at  $2\frac{1}{2}$ " slots in thrie beam expansion sections. Nuts shall fully engage bolts with a minimum of one bolt thread extending beyond the nuts. Distort the first thread on the outside of the nut to prevent loosening. Tighten guardrail bolts in  $3\frac{3}{4}$ " slots at guardrail post(s) that lie between the slotted expansion splice and bridge deck joint so that the bolt heads are in full contact with thrie-beam elements, but not so tight as to impede movement due to expansion.

NEOPRENE PADS: Neoprene pads must be plain pads with a durometer hardness of 60 or 70 and meet the requirements of Specification Section 932, except that testing of the finished pad will not be required.

ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor.

REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall conform to Section 993 of the Specifications.

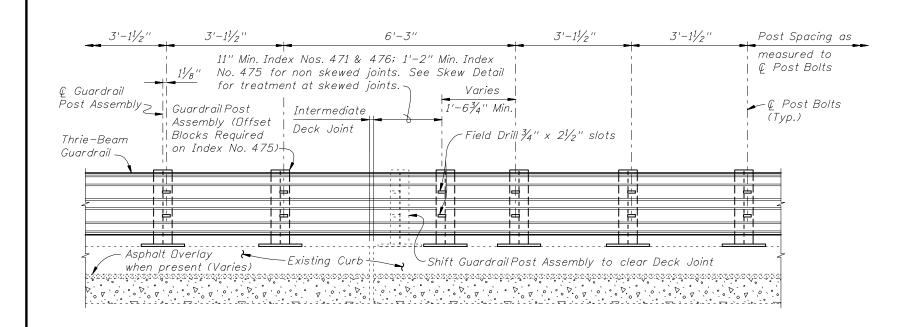
Install markers in the upper groove of the Thrie Beam Guardrail at the spacings shown in the table below. Reflector color (white or yellow) shall conform to the color of the near edgeline.

PEDESTRIAN SAFETY PIPE RAIL: Pedestrian Safety Pipe Rail is required when called for in the Plans. See Index No. 400 for details.

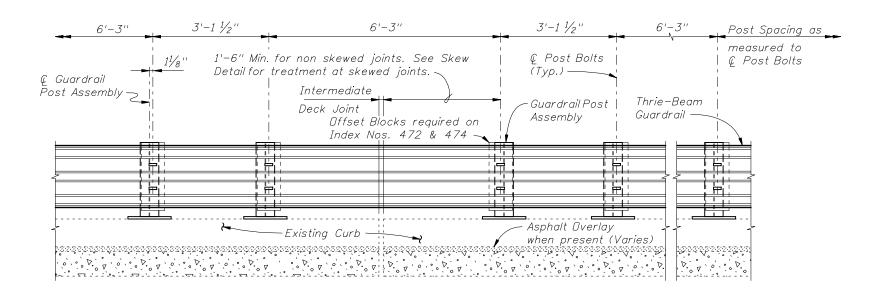
BRIDGE NAME PLATE: If a portion of the existing Traffic Railing is to be removed that carries the bridge name, number and or date, or if the installation of the Traffic Railing (Thrie Beam Retrofit) will obscure the bridge name, number and or date, then replace the information that has been removed or obscured, with 3" tall black lettering on white nonreflective sheeting applied to the top of the adjacent guardrail. The information must be clearly visible from the right side of the approaching travellane. The sheeting and adhesive backing shall comply with Specification Section 994 and may comprise of individual decals of letters and numbers.

PAYMENT: Payment will be made under Metal Traffic Railing (Thrie-Beam Retrofit) which shall include all materials and labor required to fabricate and install the barrier and lapped guardrail where necessary to maintain post spacing. The Pedestrian Safety Pipe Rail, Transition Blocks and Curbs, Bridge Name Plate, Reflective Railing Markers and installation of Elevation Markers, where required, will not be paid for directly but shall be considered as incidental work.

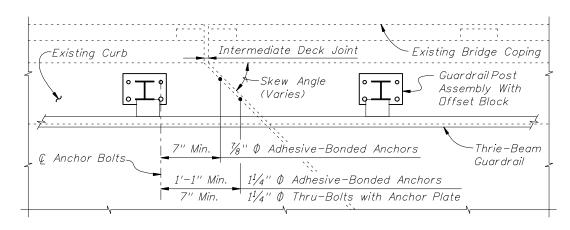
REFLECTIVE RAILING MARKER SPACING		
Distance – Edge of TravelLane to Face of Railing	Spacing (Ft.)	
< 4'	40'	
4' to 8'	80'	
> than 8'	None Required	



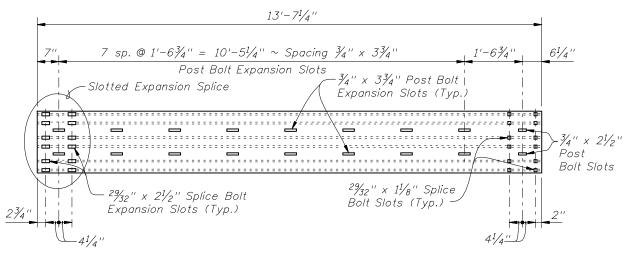
PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX NOS. 471, 475 & 476



PARTIAL ELEVATION OF INSIDE FACE OF RAILING
MODIFIED POST SPACING AT INTERMEDIATE DECK JOINTS DETAIL FOR INDEX NOS. 472, 473 & 474

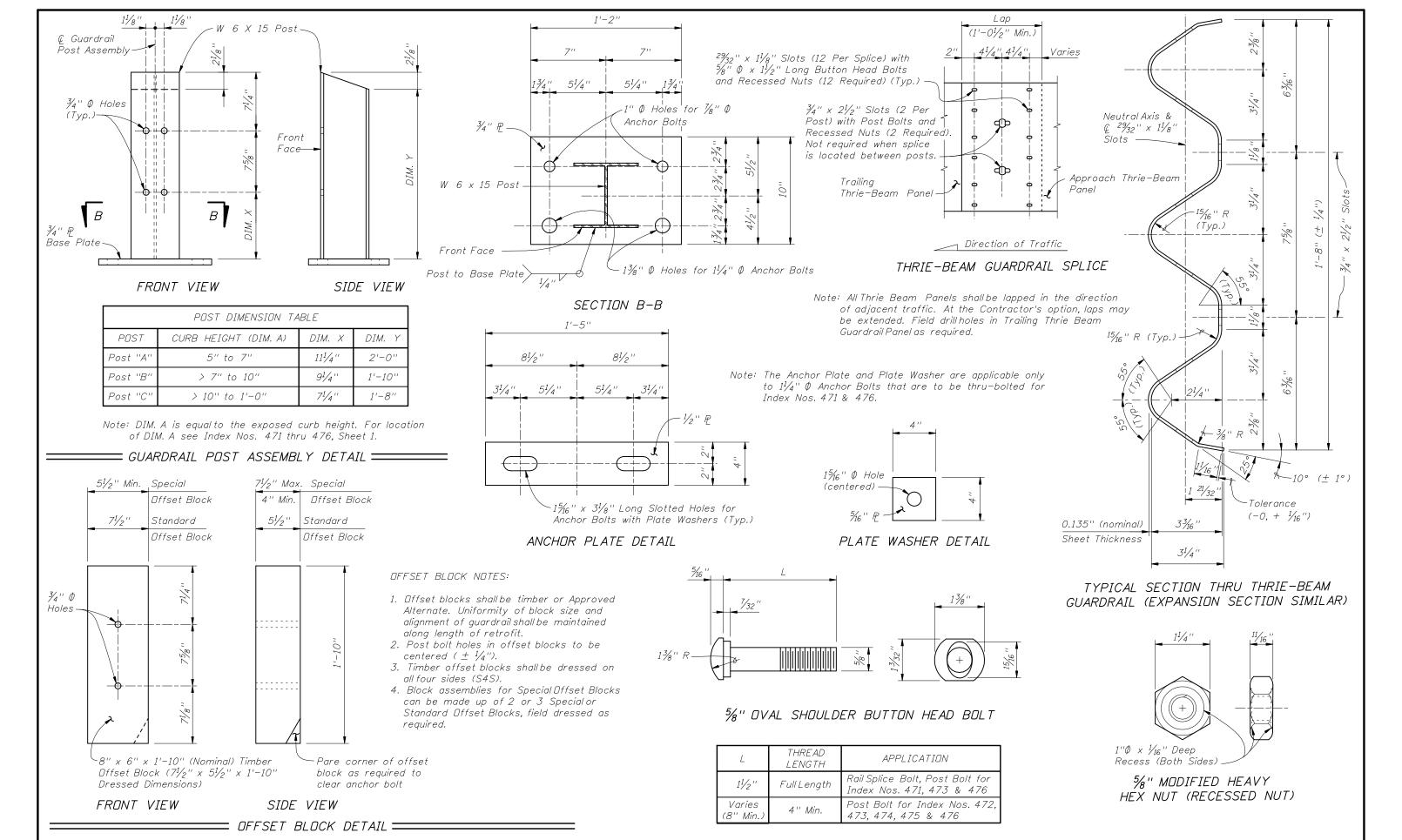


PARTIAL PLAN INTERMEDIATE JOINT SKEW DETAIL



THRIE-BEAM EXPANSION SECTION





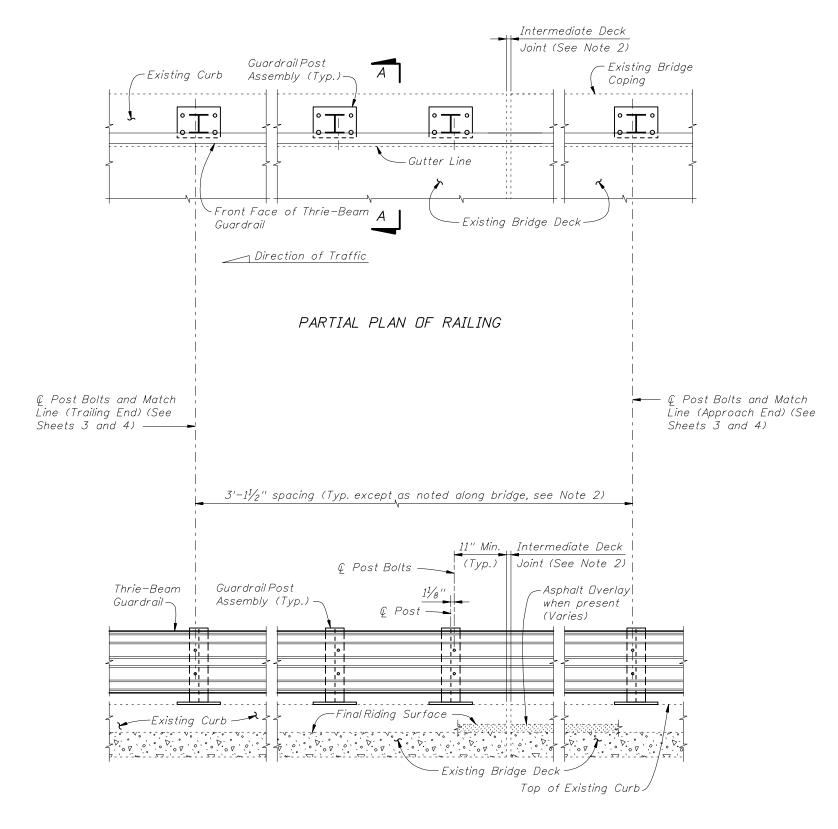


2010 FDOT Design Standards

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)
GENERAL NOTES & DETAILS

Last Revision Sheet No. 01/01/08 3 of 3

470



PARTIAL ELEVATION OF INSIDE FACE OF RAILING

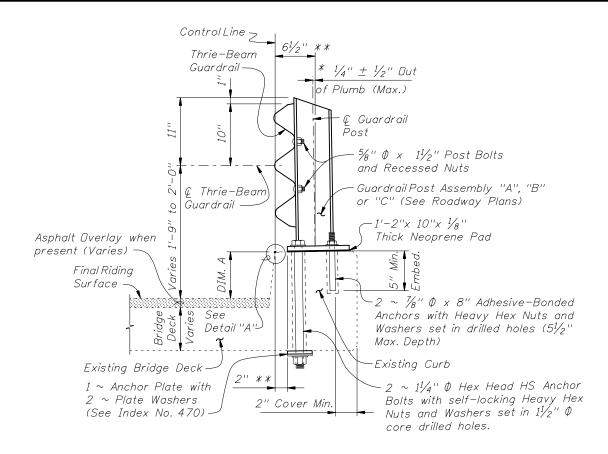
====== TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

### NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be removed off 1" below existing concrete and grouted over.

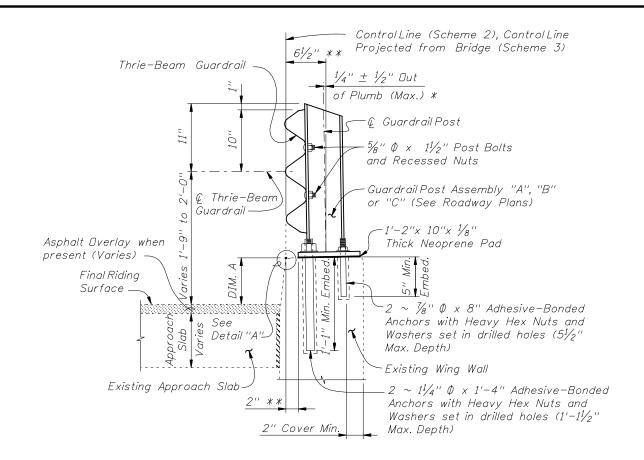
CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.





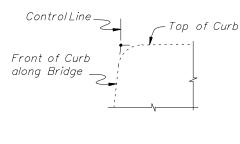
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

BILL OF REINFORCING STEEL		CING STEEL	BAR BENDING DIAGRAMS
MARK	SIZE	LENGTH	4"
Α	4	AS REQUIRED	$\sim$
D	4	1'-11''	No te
L	4	4'-1''	See 7
			<b>Dowel Bar 4D</b> (Standard 180° Hook)
_	BAR 4.4	<u> </u>	(Standard 180 Hook)
NOTES:			3-6
1 . All bar dimensions are out to out. 2. The 1'-2" vertical dimension shown for			4
Bar 4D is based on a curb height of 9". If curb height is less or more than 9", decrease or increase this dimension by an amount equal to the difference in cur- height.		or more than 9", this dimension by	DOWEL BAR 4L

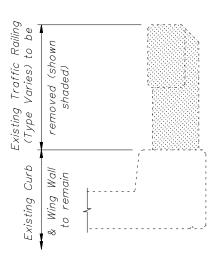


SECTION B-B
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB
(SCHEME 2 SHOWN, SCHEME 3 SIMILAR)

- \*Shim with washers around Anchors as required to maintain tolerance.
- \*\* Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



DETAIL "A"



TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

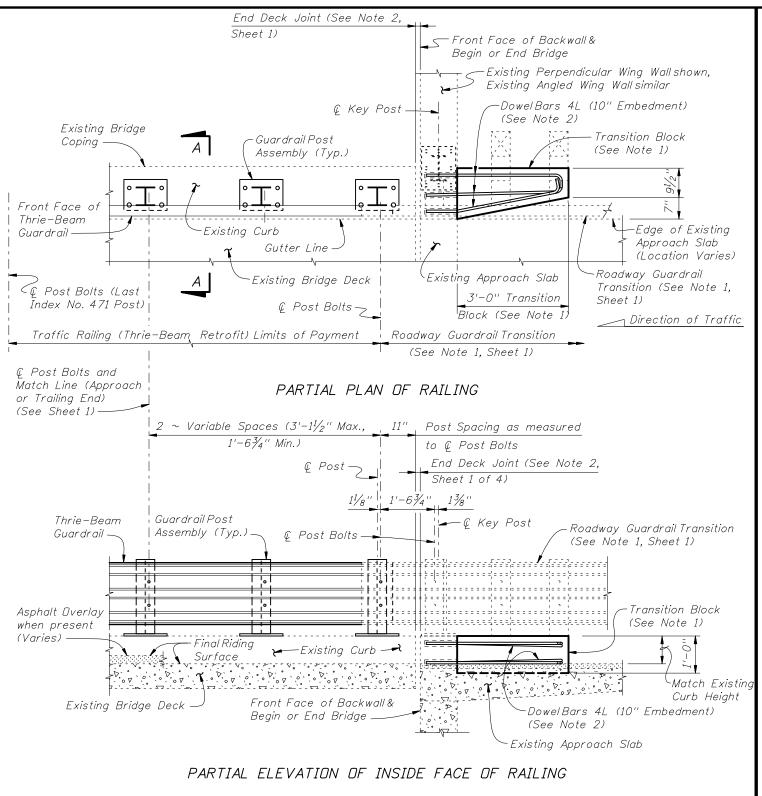
### CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.
For location of Section B-B see Sheets 3 & 4.
For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.



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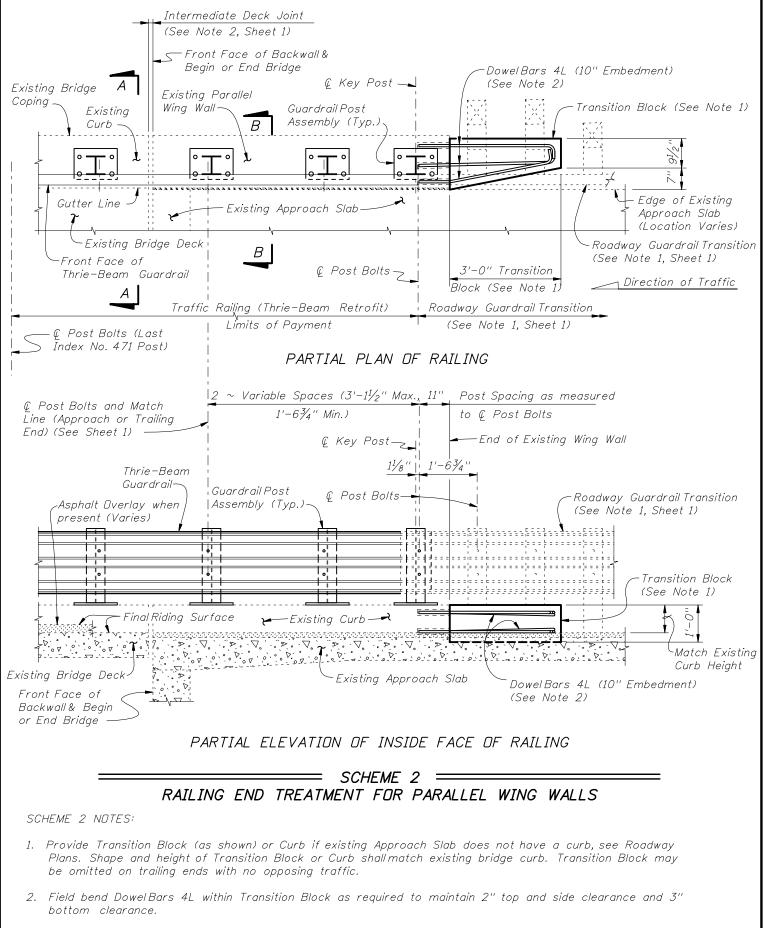
471



# RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend DowelBars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



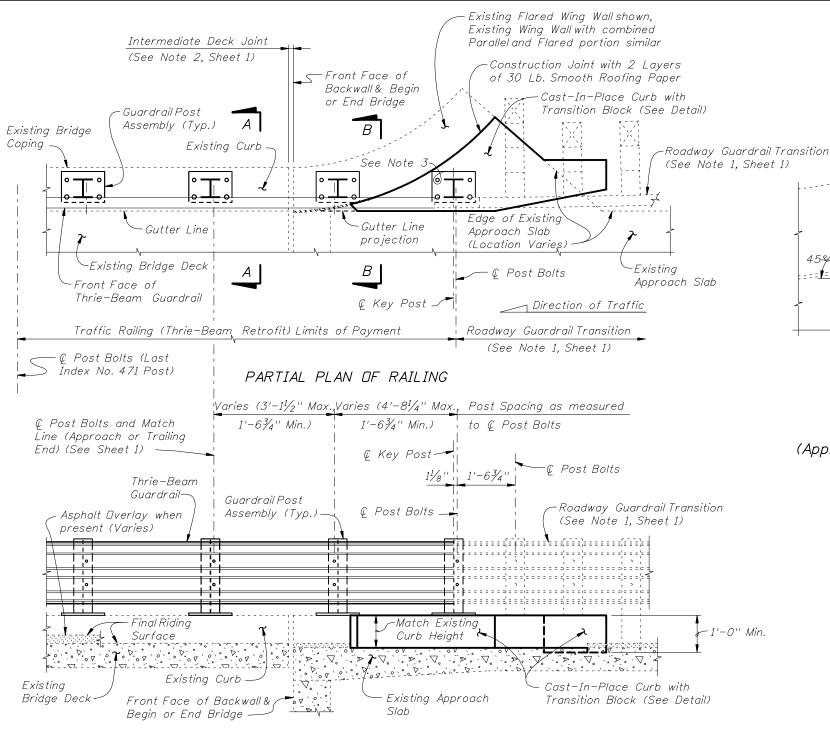


2010 FDOT Design Standards

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)
NARROW CURB

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Index No. 471



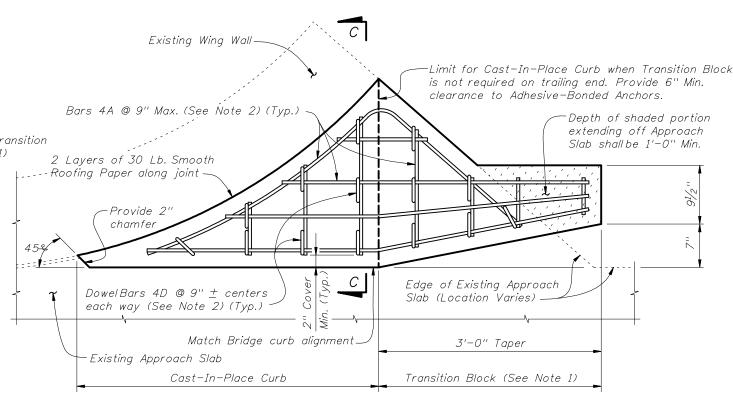
PARTIAL ELEVATION OF INSIDE FACE OF RAILING

# RAILING END TREATMENT FOR FLARED WING WALLS

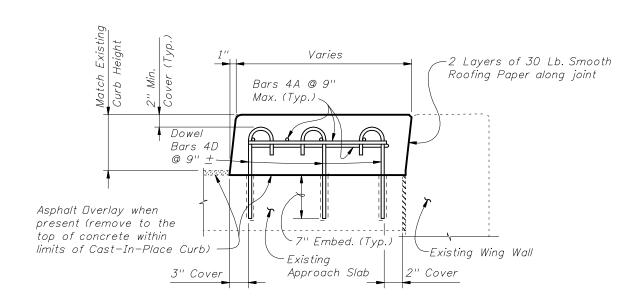
SCHEME 3 NOTES:

- 1. Provide Cast-In-Place Curb as shown. Shape and height of Transition Block and Curb shall match existing bridge curb.

  Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field cut and bend Bars 4A and rotate DowelBars 4B within Curb and Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. A single 1/8" Ø x 8" Adhesive-Bonded Anchor may be omitted as shown when 2" clear cover cannot be provided.



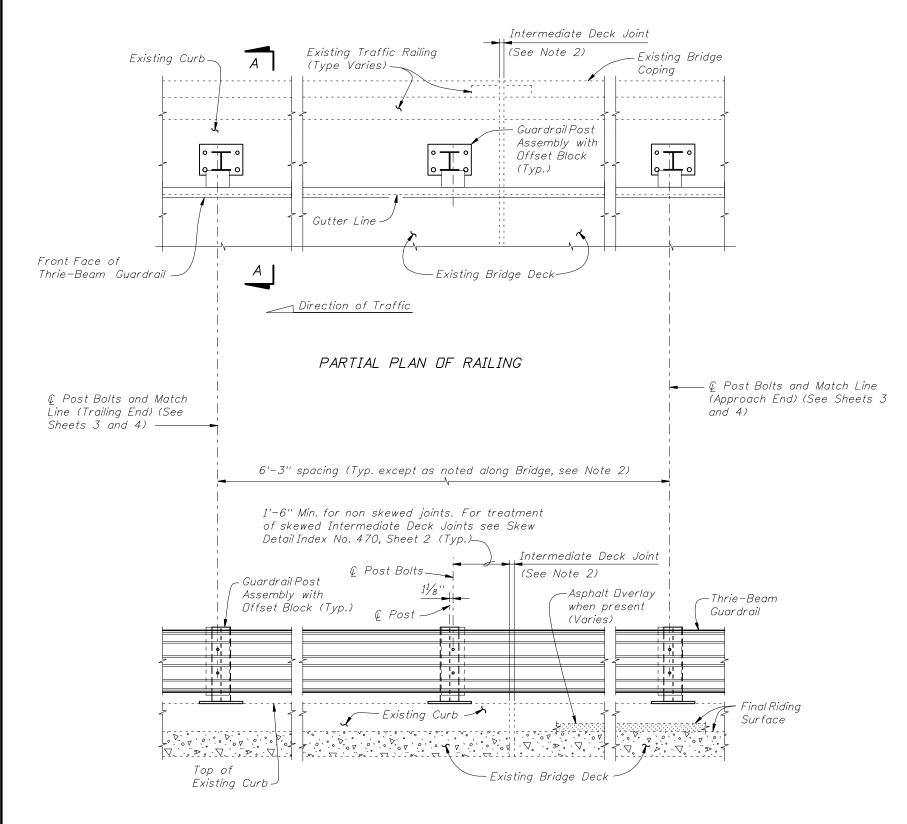
PLAN OF CAST-IN-PLACE CURB & TRANSITION BLOCK DETAIL (Approach End with Transition Block Shown, Trailing End without Transition Block Similar)



SECTION C-C



Sheet No.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

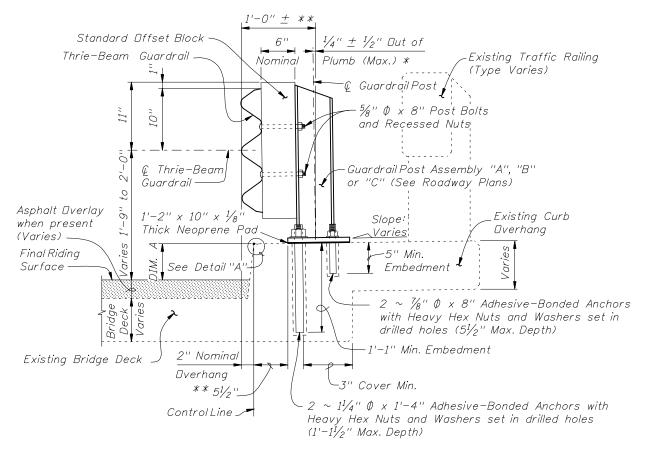
TYPICAL TREATMENT OF RAILING ALONG BRIDGE

### NOTES:

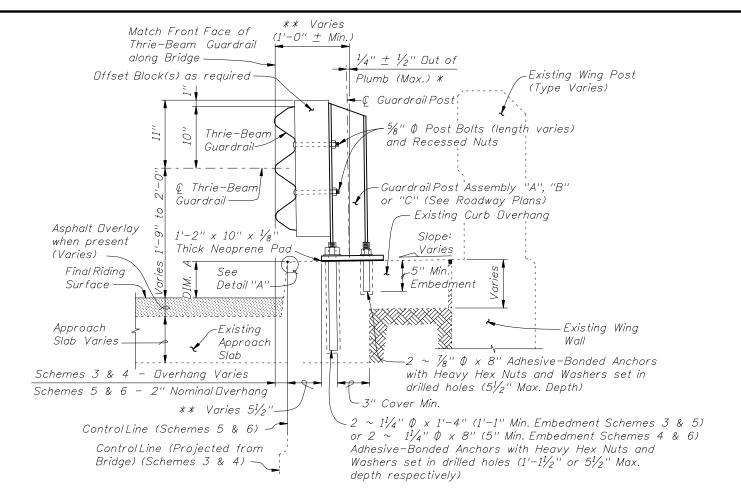
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.

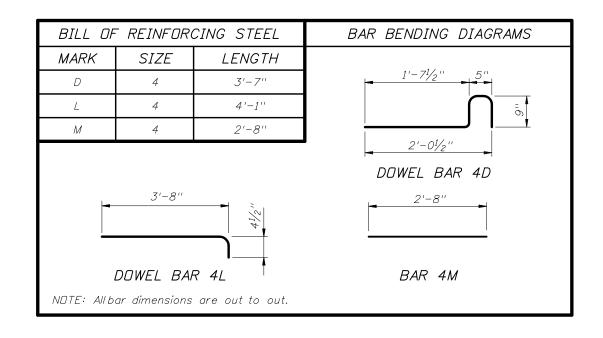


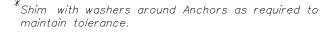


SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

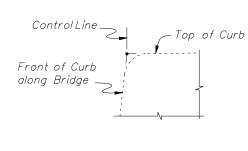


SECTION B-B
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB
(SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

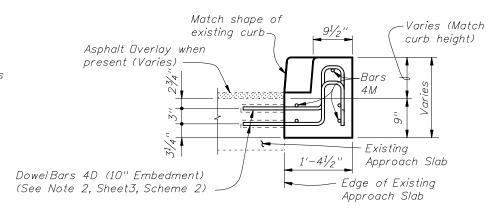




\*\* Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.



DETAIL "A"



VIEW C-C

### CROSS REFERENCES:

For location of Section A-A see Sheets 1, 3 & 4.

For location of Section B-B see Sheet 4.

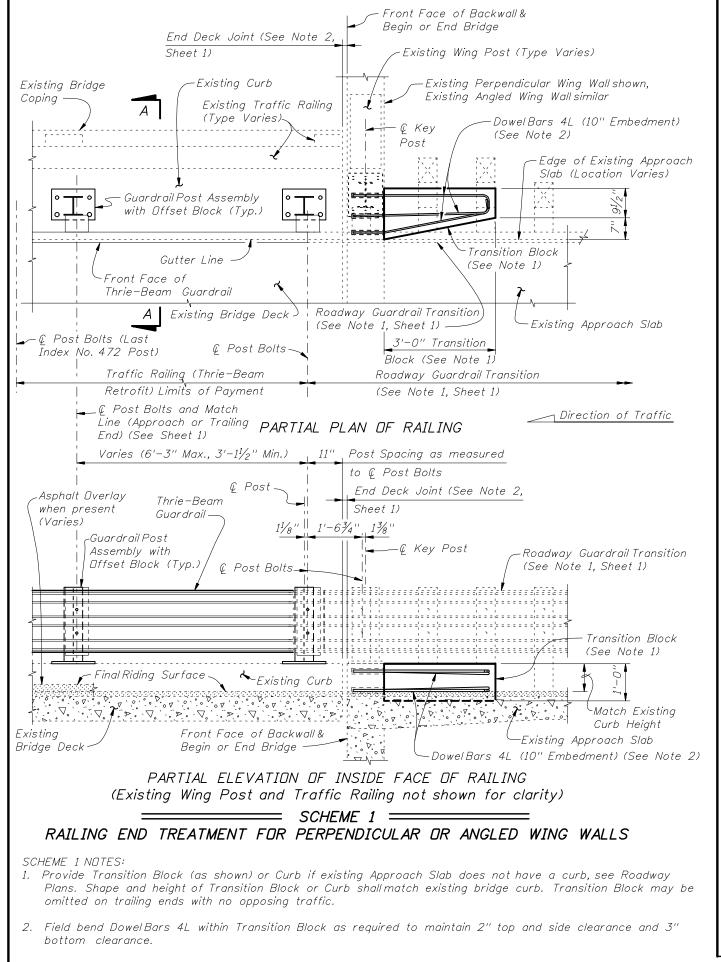
For location of View C-C see Sheet 3.

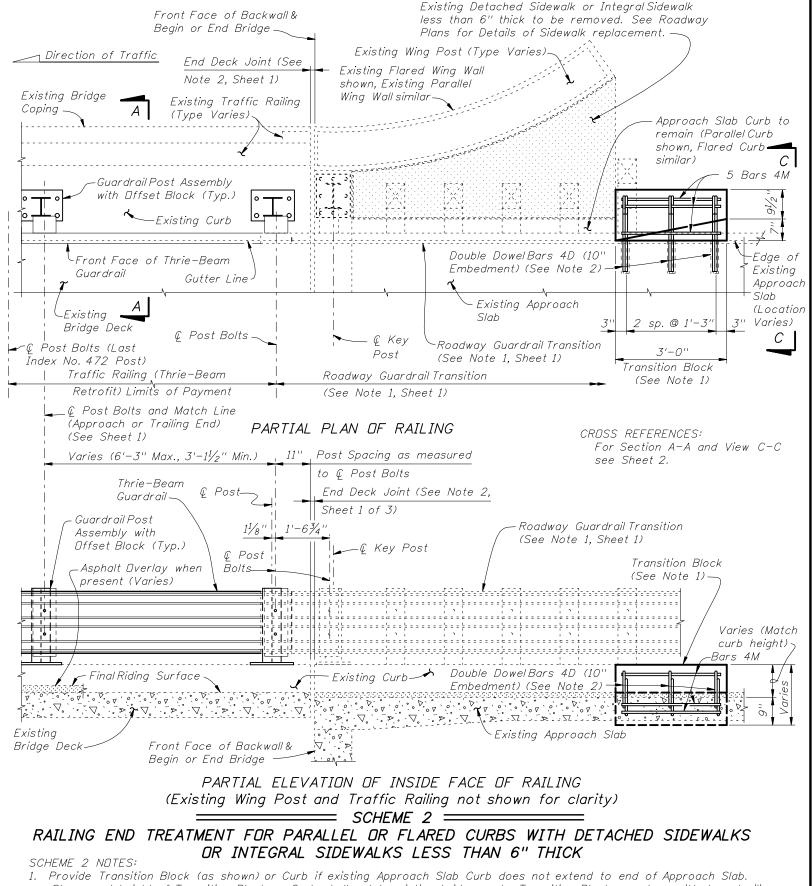
For application of Dim. A see Post Dimension Table

on Index 470, Sheet 3.



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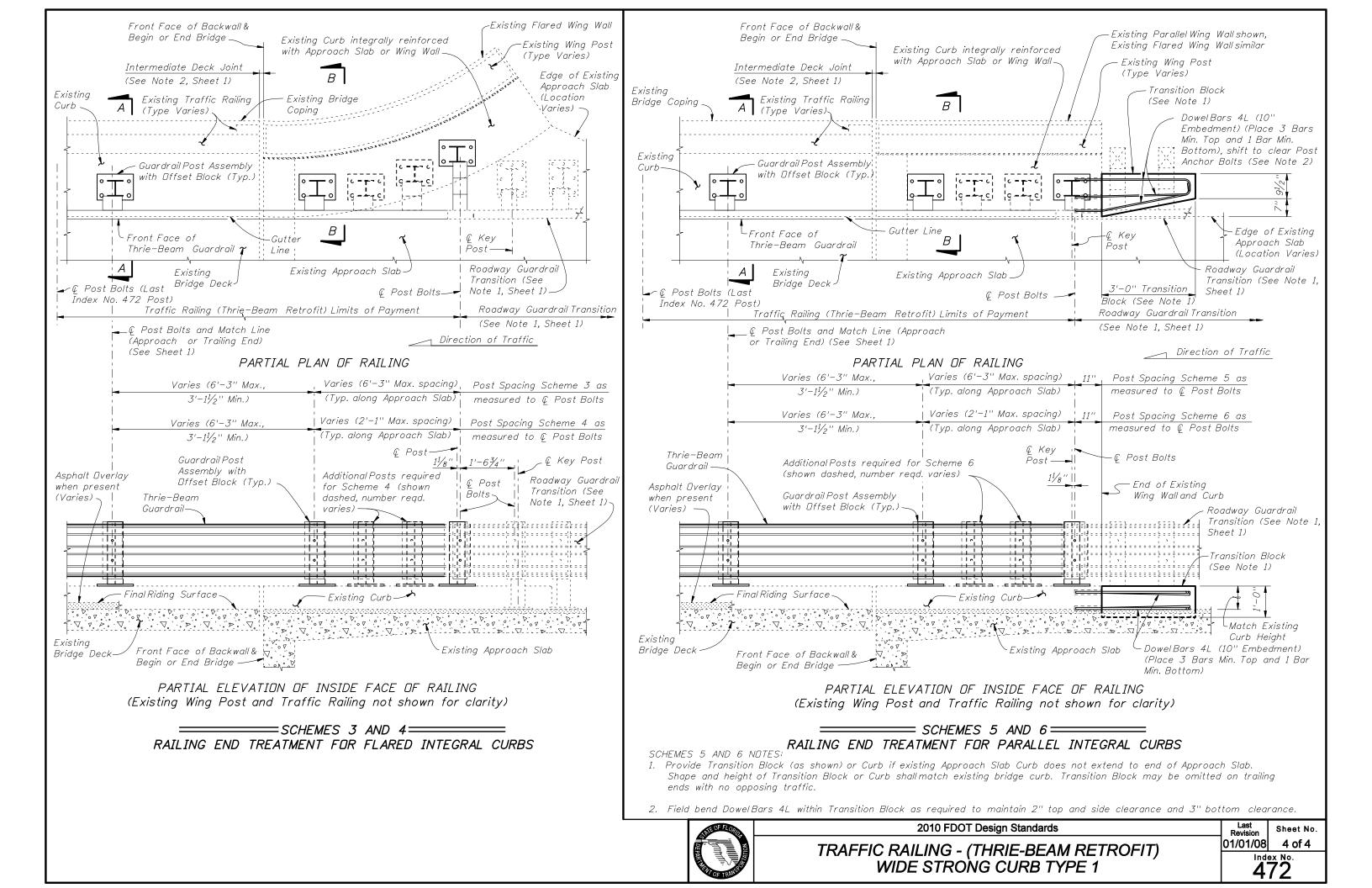


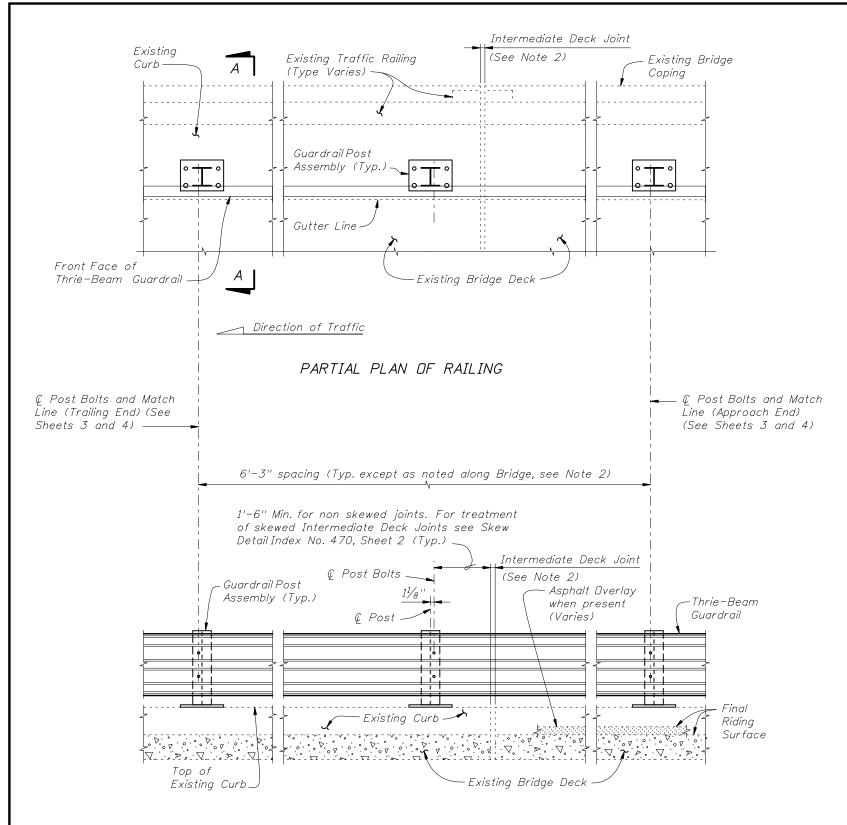
- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs.
- 2. Field bend or tilt DowelBars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



2010 FDOT Design Standards

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)
WIDE STRONG CURB TYPE 1





PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

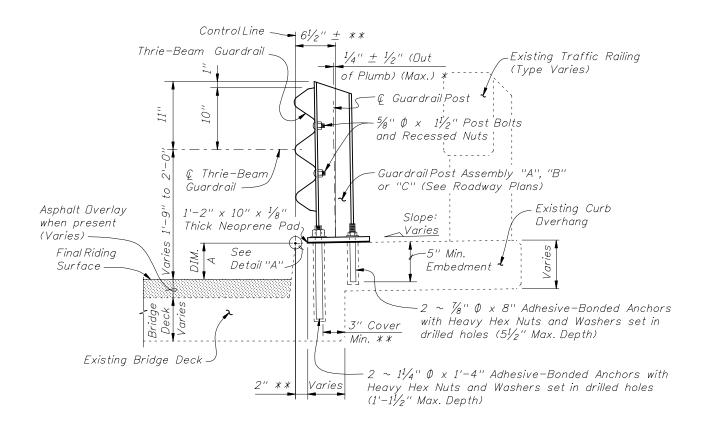
TYPICAL TREATMENT OF RAILING ALONG BRIDGE

### NOTES:

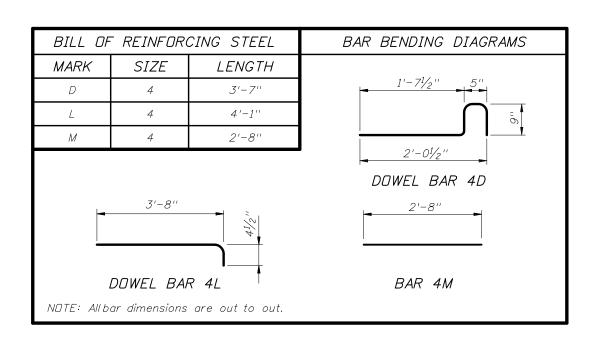
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

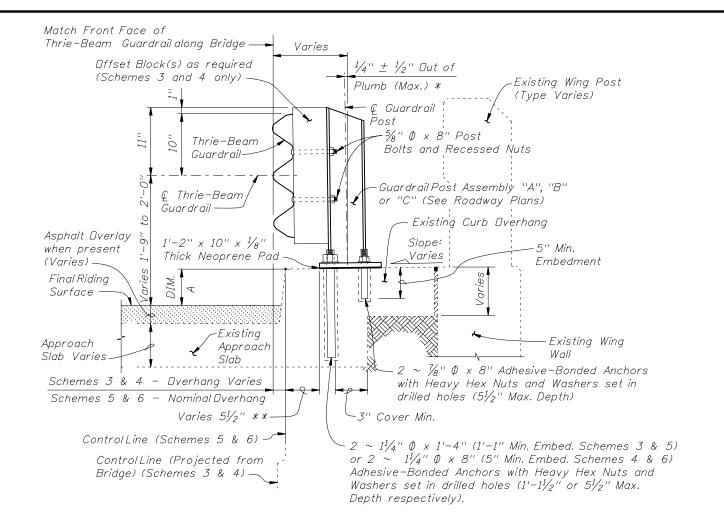
CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.





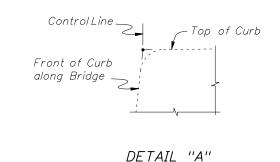
SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

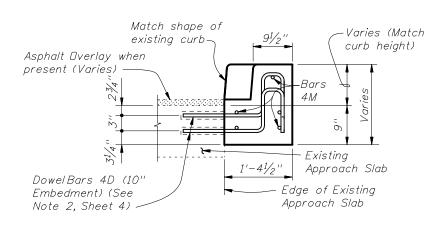




SECTION B-B
TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB
(SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

- \* Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.
- \*\* Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors and Anchor Bolts to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

# CROSS REFERENCES:

For location of Section A-A see Sheet 1, 3 and 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For Traffic Railing Notes and Details see Index No. 470.

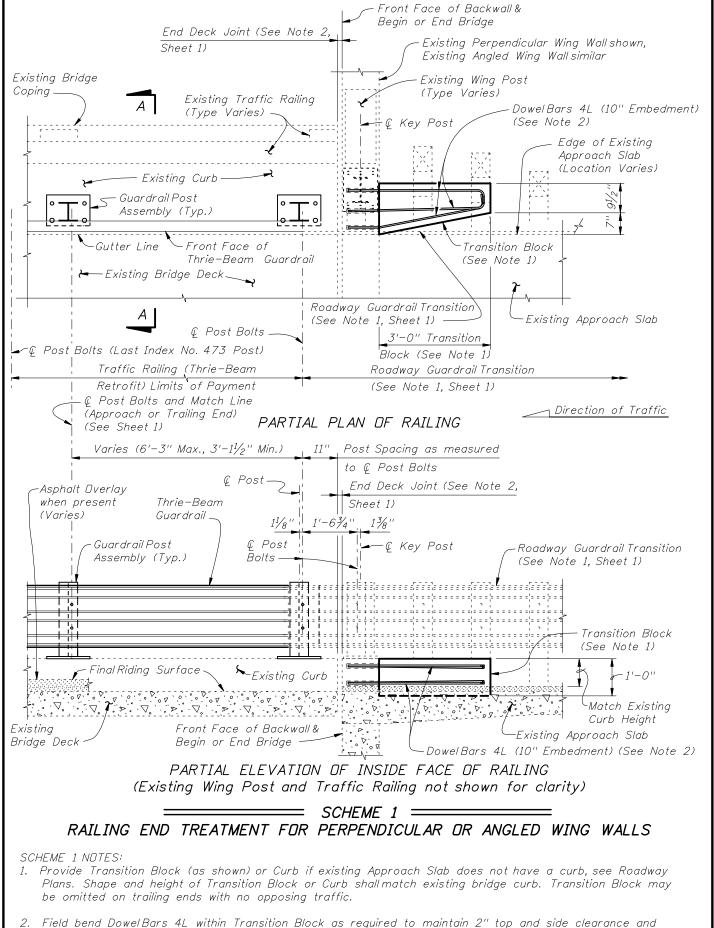
For application of Dim. A see Post Dimension Table

on Index 470, Sheet 3.

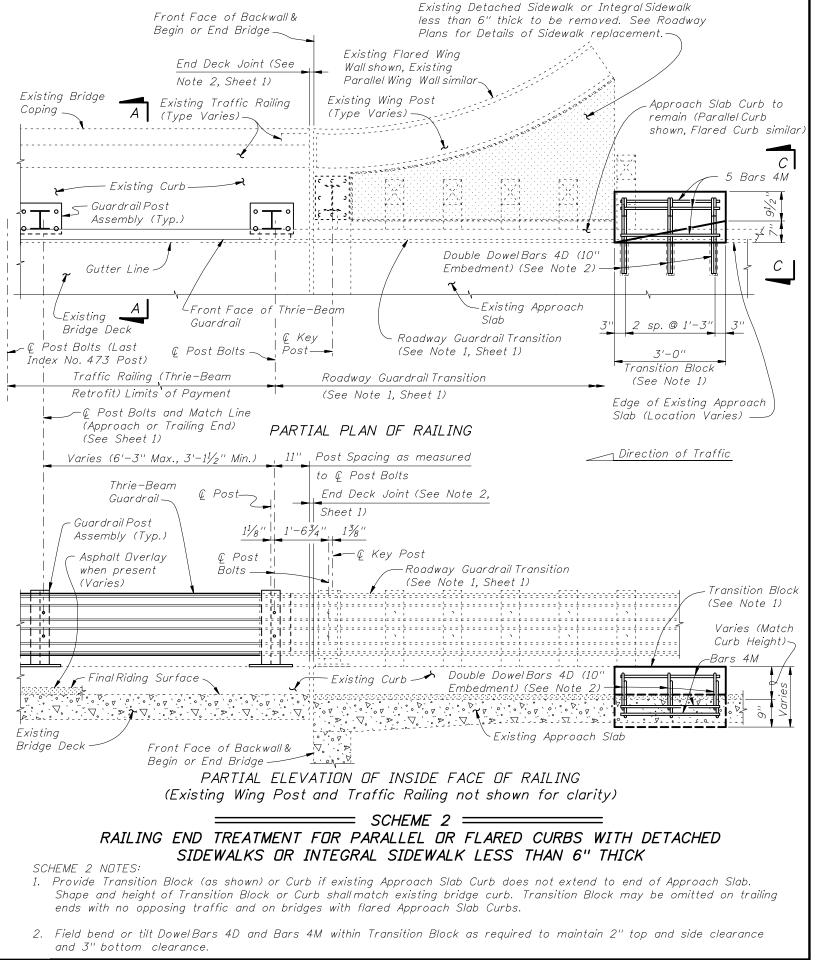
DEPARTMENT OF TRANSPORT

### 2010 FDOT Design Standards

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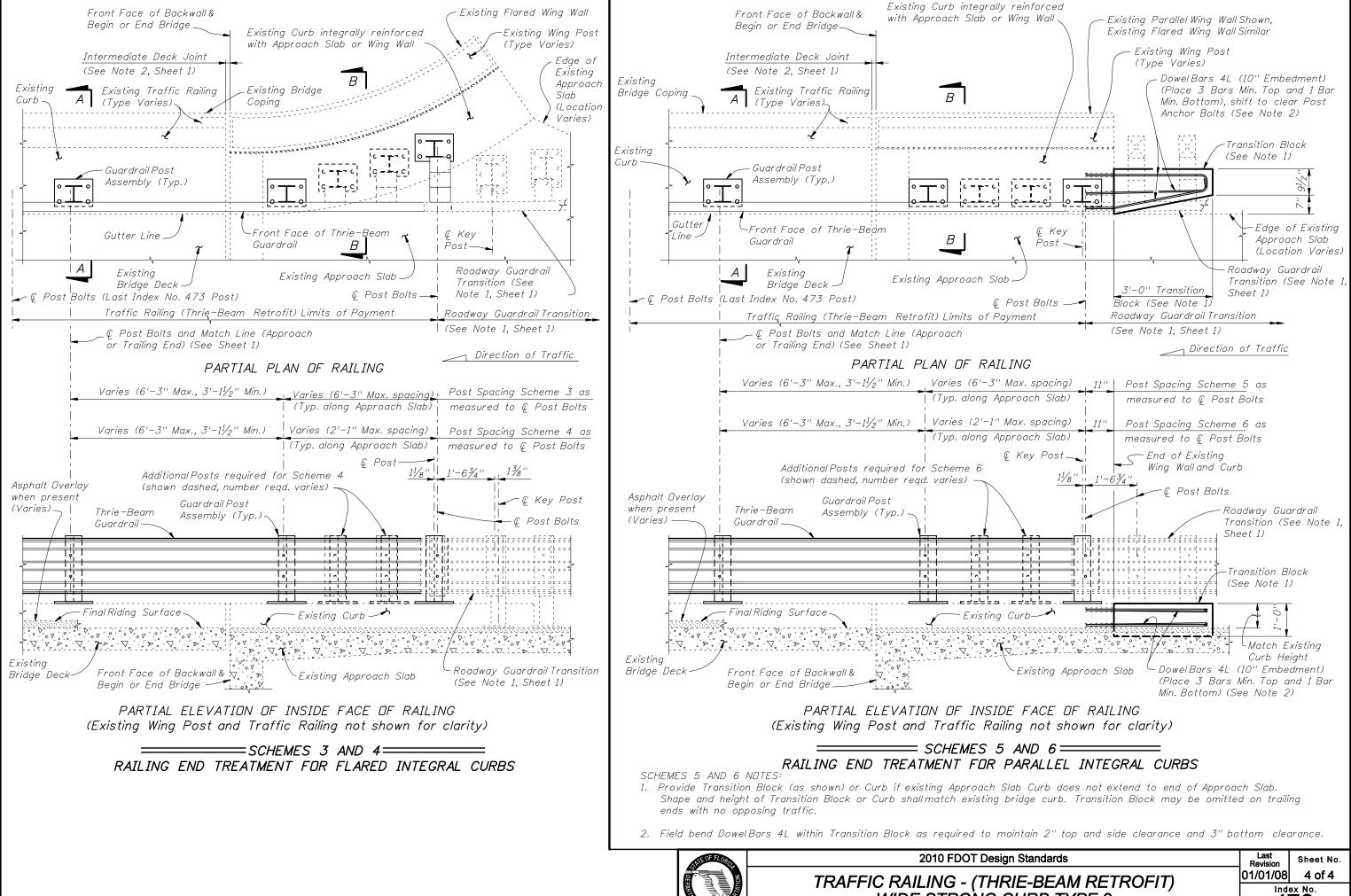
2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



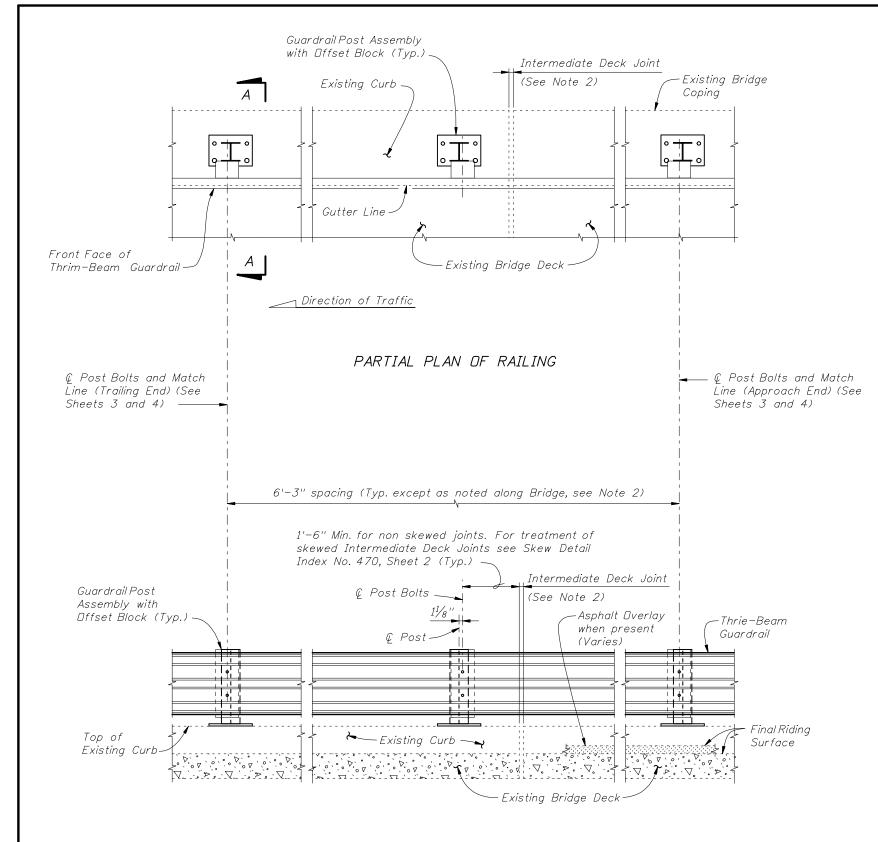
2010 FDOT Design Standards

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TRAFFIC RAILING - (THRIE-BEAM RETROFIT) **WIDE STRONG CURB TYPE 2** 



**WIDE STRONG CURB TYPE 2** 



PARTIAL ELEVATION OF INSIDE FACE OF RAILING

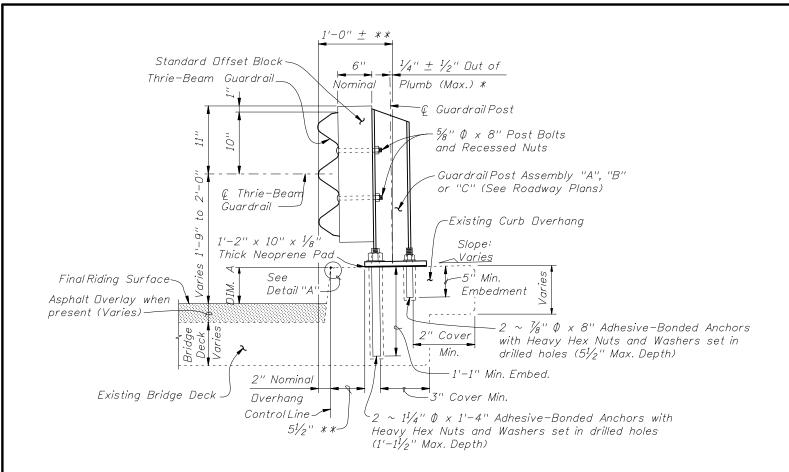
TYPICAL TREATMENT OF RAILING ALONG BRIDGE

### NOTES:

- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

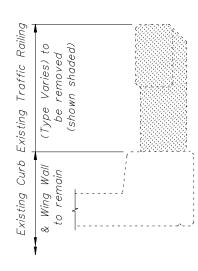
CROSS REFERENCES:
For Match Line see Sheets 3 & 4.
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details see
Index No. 470.



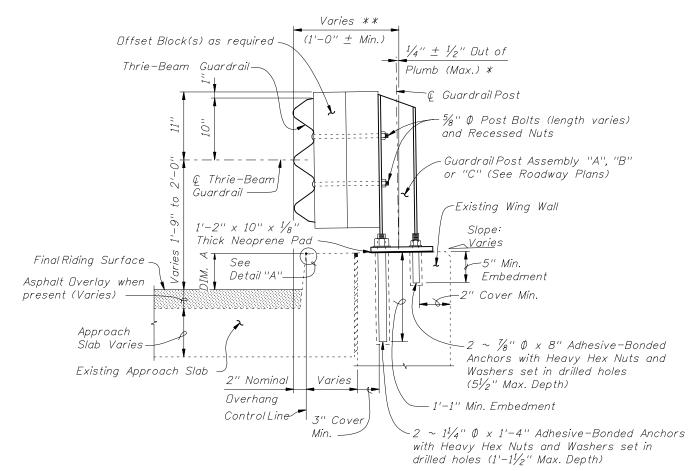


SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK

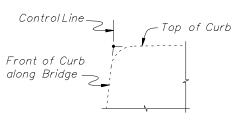
BILL OF REINFORCING STEEL				
MARK	SIZE	LENGTH		
L	4	4'-1''		
BAR BENDING DIAGRAM				
3'-8"				
DOWEL BAR 4L				
NOTE: All bar dimensions are out to out.				



TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



# SECTION B-B (SCHEME 2) TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB



DETAIL "A"

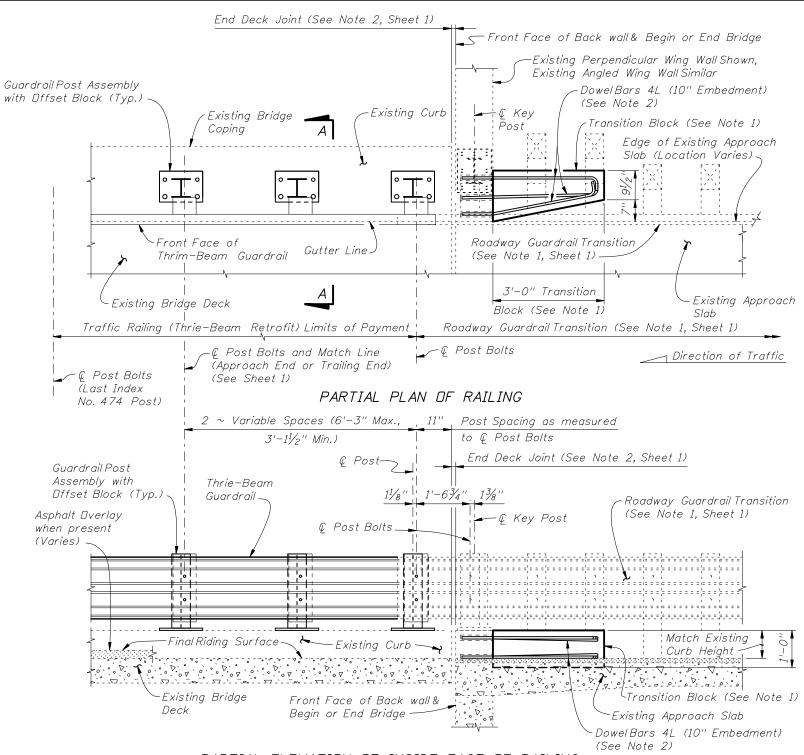
### CROSS REFERENCES:

For location of Section A-A see Sheet 1 and 3.
For location of Section B-B see Sheet 3
For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.



<sup>\*</sup>Shim with washers around Anchor Bolts and Anchors as required to maintain tolerance.

<sup>\*\*</sup>Dffset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.

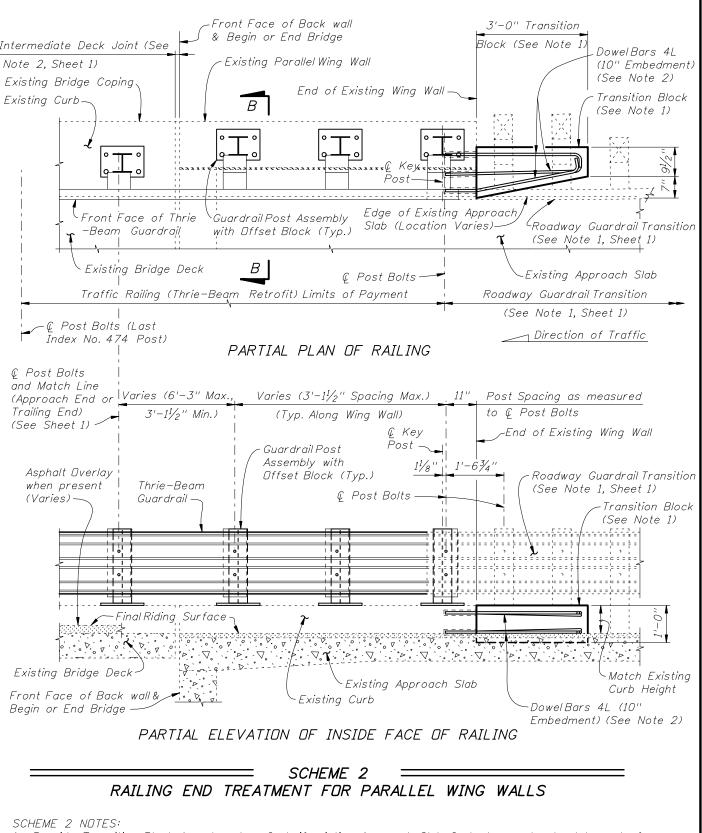


PARTIAL ELEVATION OF INSIDE FACE OF RAILING

# RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

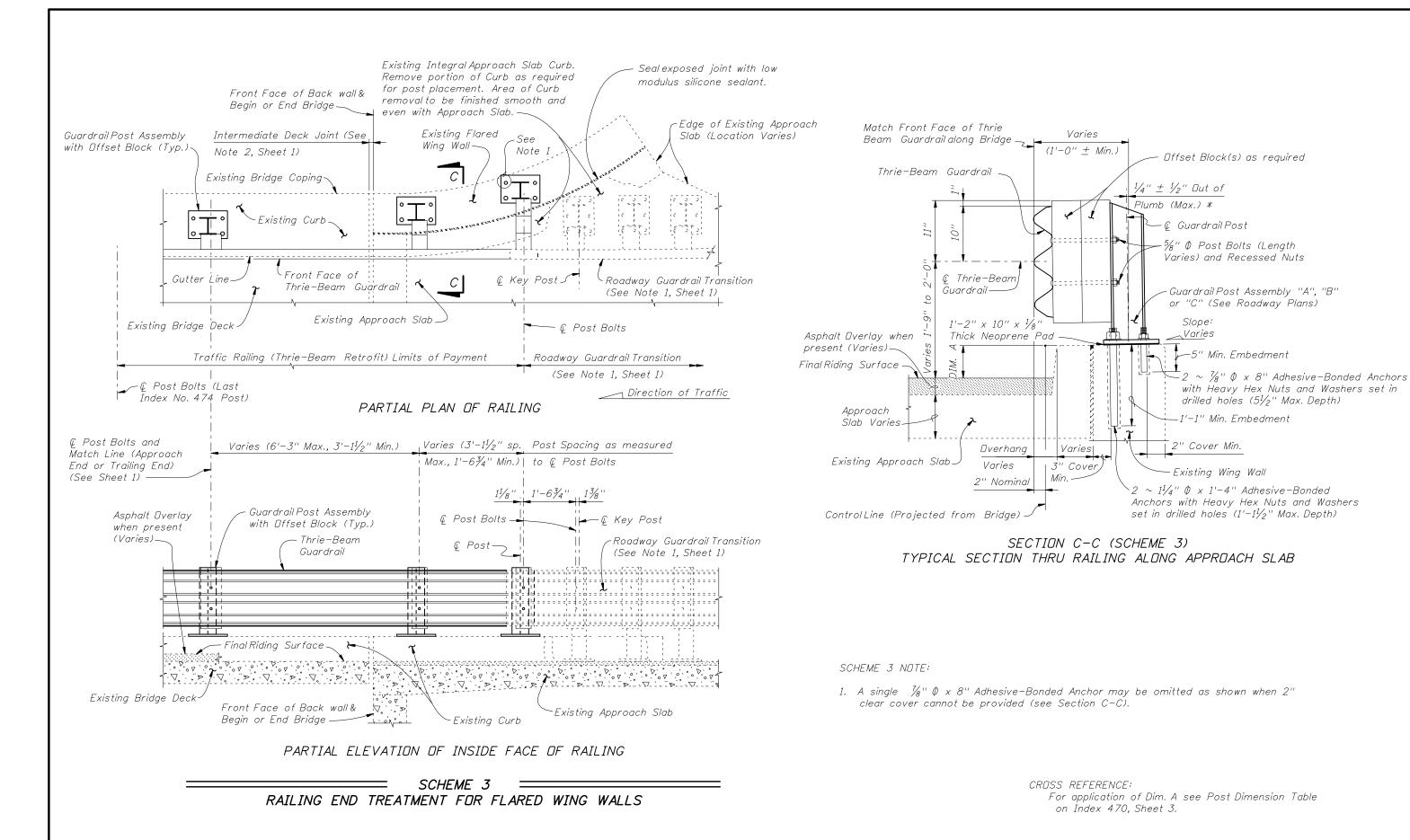
SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.





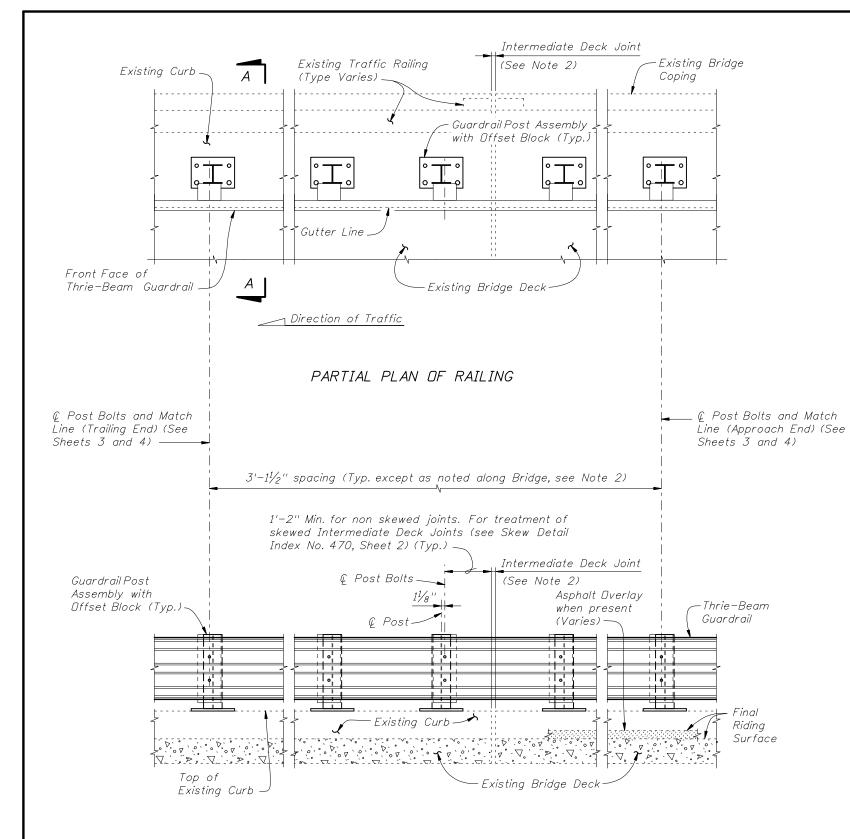
2010 FDOT Design Standards

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)

INTERMEDIATE CURB

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Index No. 474



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

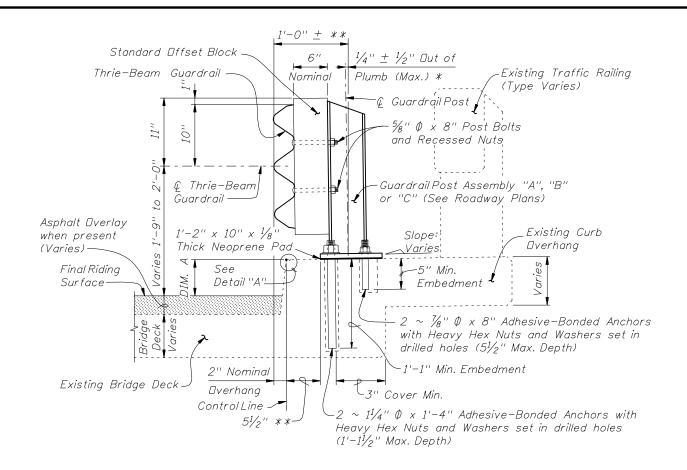
==== TYPICAL TREATMENT OF RAILING ALONG BRIDGE ======

#### NOTES:

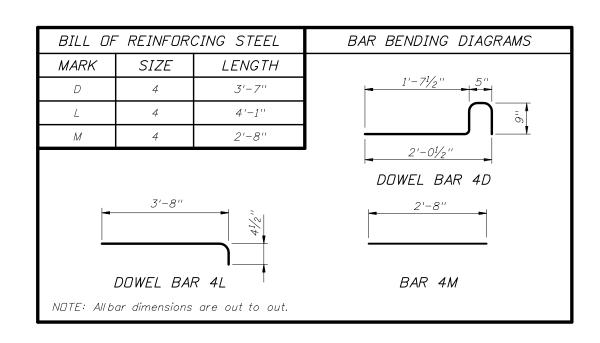
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CRUSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.





SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

when present

Final Riding

Approach

Slab Varies

Schemes 3 & 4 '- Overhang Varies

Schemes 5 & 6 - 2" Nominal Overhang

ControlLine (Schemes 5 & 6) ~

ControlLine (Projected from

Bridge) (Schemes 3 & 4) —

Surface

(Varies)

Thrie-Beam Guardrail along Bridge

Offset Block(s) as required -

Thrie-Beam

Guardrail ·

¢ Thrie−Beam

 $1'-2'' \times 10'' \times \frac{1}{8}$ 

See

Thick Neoprene Pad

Detail ''A'

Existing

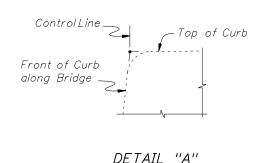
Approach

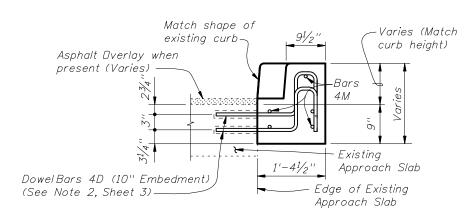
Slab

Varies 51/2" \*\*

Guardrail ·

<sup>\*\*</sup> Offset may vary ± 1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

#### CROSS REFERENCES:

Varies \*\*

 $(1'-0'' \pm Min.)$ 

 $\frac{1}{4}$ "  $\pm \frac{1}{2}$ " Dut of

-Ç Guardrail Post¦

5%'' ∅ Post Bolts (length varies)

-Guardrail Post ˈAssembly ''A'', ''B''

Existing Curb Overhang

or "C" (See Roadway Plans)

and Recessed Nuts

Varies

Embedment

-5" Min

∽3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively)

Plumb (Max.) \*

-Existing Wing Post

Existing Wing

~ \( \gamma'' \psi \times 8'' \) Adhesive-Bonded Anchors

 $\sim 1^{1}/_{4}$ "  $\emptyset \times 1'-4$ " (1'-1" Min. Embed. Schemes 3 & 5)

or  $2 \sim 1^{1}/_{4}$ "  $\emptyset \times 8$ " (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes  $(1'-1^{1/2})''$  or  $5^{1/2}$ '' Max.

with Heavy Hex Nuts and Washers set in

drilled holes  $(5\frac{1}{2}"$  Max. Depth)

(Type Varies)

For location of Section A-A see Sheet 1, 3 & 4.

For location of Section B-B see Sheet 4.

For location of View C-C see Sheet 3.

For application of Dim. A see Post Dimension Table on Index 470, Sheet 3.



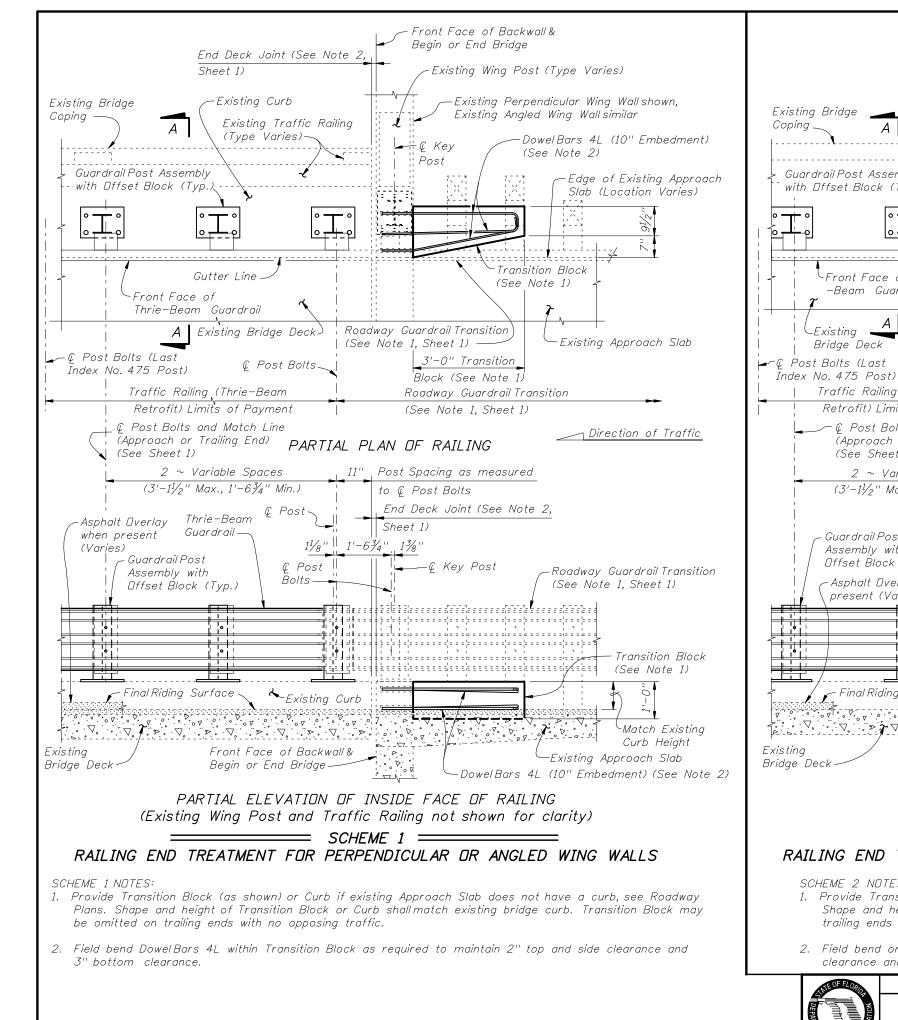
2010 FDOT Design Standards

**WIDE CURB TYPE 1** 

TRAFFIC RAILING - (THRIE-BEAM RETROFIT)

Last Sheet No. 07/01/08 2 of 4

1ndex No. 475



(Existing Wing Post and Traffic Railing not shown for clarity) \_\_\_\_\_\_\_ SCHEME 2 == RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALKS LESS THAN 6" THICK SCHEME 2 NOTES:

PARTIAL ELEVATION OF INSIDE FACE OF RAILING

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab. Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs.
- 2. Field bend or tilt DowelBars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance



Front Face of Backwall &

End Deck Joint (See

Note 2, Sheet 1)

Gutter Line

(Approach or Trailing End) PARTIAL PLAN OF RAILING

@ Post Bolts \_

@ Post-

Front Face of Backwall&

Begin or End Bridge-

Post

Thrie-Beam

Existing Traffic Railing

(Type Varies)-

Guardrail Post Assembly

with Dffset Block (Typ.)

-Existing

Bridge Deck

Front Face of Thrie

Traffic Railing (Thrie-Beam

(See Sheet 1)

Assembly with

Offset Block (Typ.)

present (Varies)

-Asphalt Overlay when

Retrofit) Limits of Payment

2 ~ Variable Spaces

 $(3'-1\frac{1}{2}'')$  Max.,  $1'-6\frac{3}{4}''$  Min.)

Guardrail Post Guardrail-

Post Bolts and Match Line

-Beam Guardrail

Begin or End Bridge.



Existing Detached Sidewalk or Integral Sidewalk

Plans for Details of Sidewalk replacement.

Double Dowel Bars 4D (10"

Embedment) (See Note 2)

-Roadway Guardrail Transition

Slab

(See Note 1, Sheet 1)

Roadway Guardrail Transition

Existing Approach Slab

Embedment) (See Note 2)

(See Note 1, Sheet 1)

Existing Curb 
Double Dowel Bars 4D (10"

Existing Approach

Existing Wing Post (Type Varies) -

Existing Flared Wing Wall

shown, Existing Parallel

Wing Wall similar

Q Key

(See Note 1, Sheet 1)

11" Post Spacing as measured

to & Post Bolts

Sheet 1)

1'-63/4''

Roadway Guardrail Transition

End Deck Joint (See Note 2,

-@ Key Post

Post

less than 6" thick to be removed. See Roadway

Approach Slab Curb to

5 Bars 4M

Edae of

Existing

Approach

(Location

Varies)

Slab

Transition Block (See Note 1)

Varies (Match Curb Height)~

-Bars 4M

remain (Parallel Curb

shown, Flared Curb

similar)

2 sp. @ 1'-3'

3'-0"

(See Note 1)

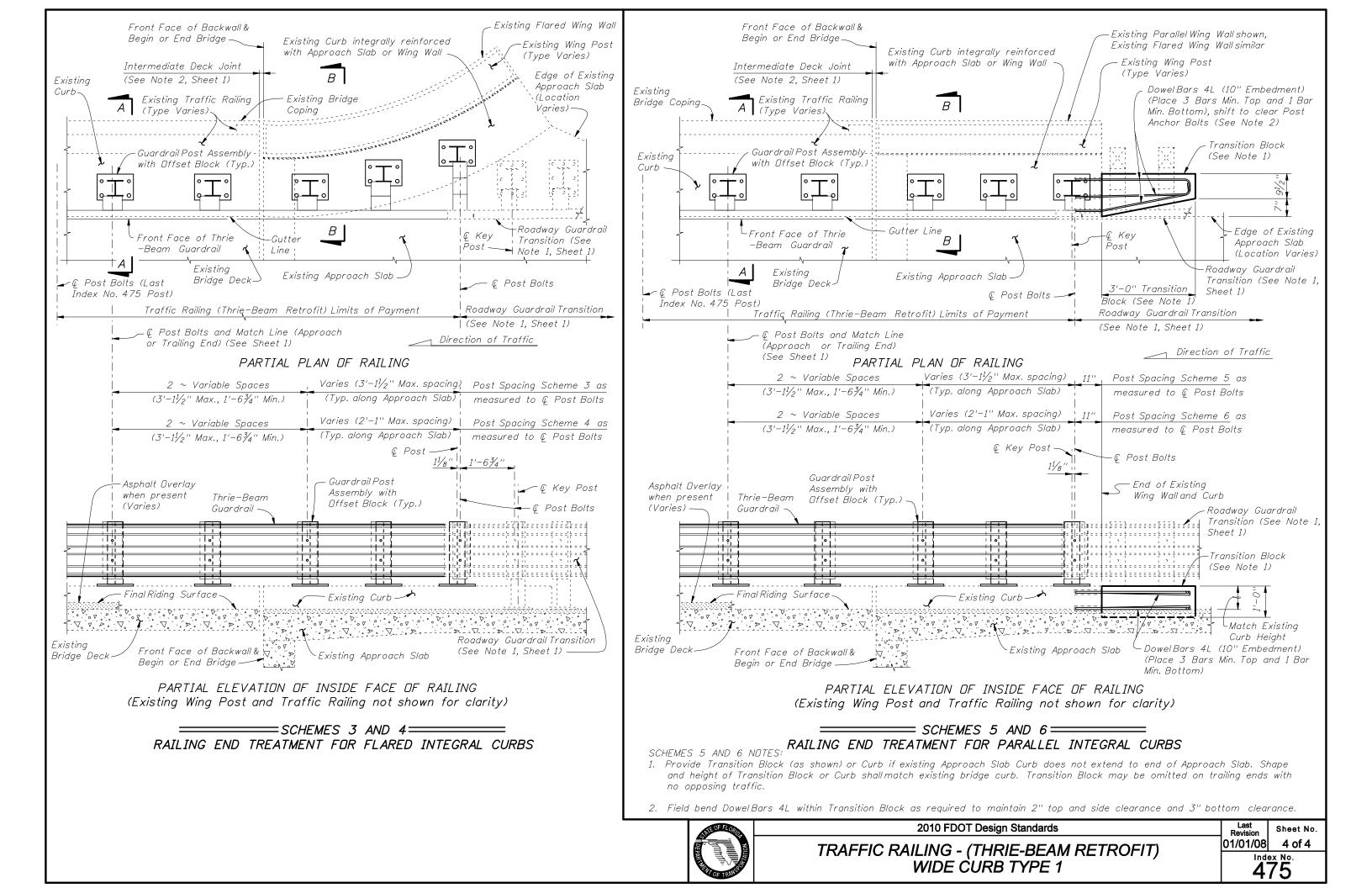
→ Direction of Traffic

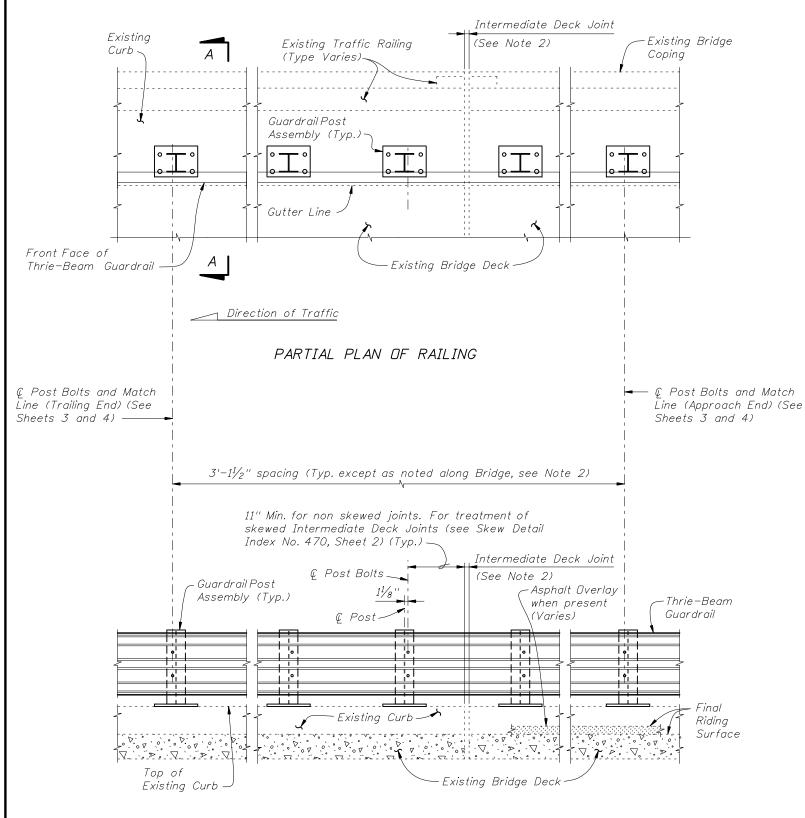
Transition Block

01/01/08 3 of 4 475

Sheet No.

TRAFFIC RAILING - (THRIE-BEAM RETROFIT) **WIDE CURB TYPE 1** 





PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Traffic Railing not shown for clarity)

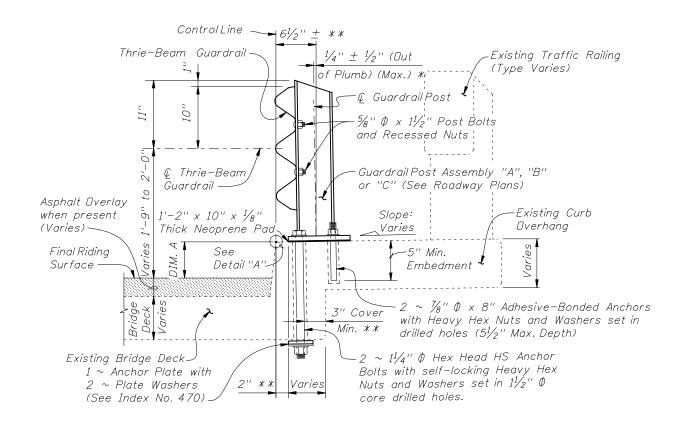
= TYPICAL TREATMENT OF RAILING ALONG BRIDGE ====

#### NOTES:

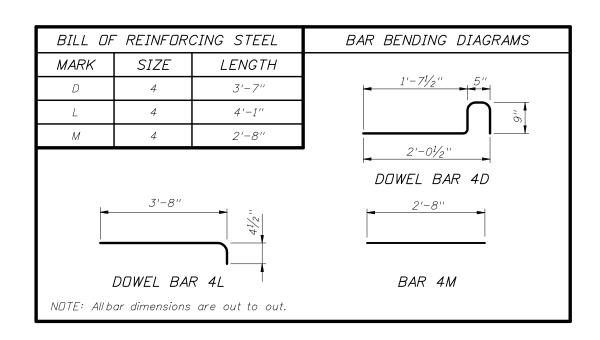
- 1. On approach end provide Index No. 402 (as shown) or other site specific treatment, see Roadway Plans. For treatment of trailing end see Roadway Plans.
- 2. Actual joint dimension and orientation vary. For Intermediate Deck Joints use the Modified Post Spacing at Intermediate Deck Joints Detail, Index No. 470, Sheet 2, as required.
- 3. Areas where existing structure has been removed shall match adjoining areas and shall be finished flat by grouting or grinding as required. Exposed existing reinforcing steel shall be burned off 1" below existing concrete and grouted over.

CROSS REFERENCES:
For Section A-A see Sheet 2.
For Traffic Railing Notes and Details
see Index No. 470.





SECTION A-A TYPICAL SECTION THRU RAILING ON BRIDGE DECK





Match Front Face of

Asphalt Overlay

when present

Final Riding

Surface

Approach

Slab Varies

(Varies)-

Thrie-Beam Guardrail along Bridge S

Offset Block(s) as required

(Schemes 3 and 4 only) -

Thrie-Beam

Guardrail ·

¢ Thrie−Beam

1'-2" x 10" x 1/8"

Thick Neoprene Pad

-Existing

Approach

Varies 51/2" \*\*

Guardrail

Schemes 3 & 4 - Overhang Varies

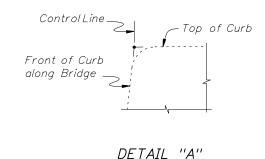
Schemes 5 & 6 - Nominal Dverhang

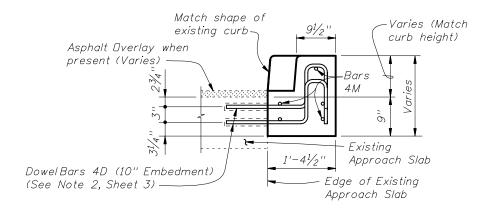
ControlLine (Schemes 5 & 6) ~

ControlLine (Projected from

Bridge) (Schemes 3 & 4)—

<sup>\*\*</sup> Offset may vary  $\pm$  1" for Adhesive-Bonded Anchors to clear existing curb reinforcing and provide minimum edge clearance. Offset shall be consistent along length of bridge.





VIEW C-C

#### CROSS REFERENCES:

For location of Section A-A see Sheet 1. 3 & 4.

For location of Section B-B see Sheet 4.

 $\frac{1}{4}$ "  $\pm \frac{1}{2}$ " Dut of

© Guardrail Post

5%" Ø x 8" Post

Slope:

3" Cover Min.

SECTION B-B

TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEMES 5 AND 6 SHOWN, SCHEMES 3 AND 4 SIMILAR)

Depth respectively).

Varies !

Bolts and Recessed Nuts

Guardrail Post Assembly "A", "B"

- Existing Curb Overhang

\_5" Min.

drilled holes  $(5\frac{1}{2}"$  Max. Depth)

Embedment

~ 78" Ø x 8" Adhesive-Bonded Anchors

with Heavy Hex Nuts and Washers set in

 $2 \sim 1^{1}/_{4}$ "  $\emptyset \times 1'-4$ " (1'-1" Min. Embed. Schemes 3 & 5) or 2  $\sim$  1 $\frac{1}{4}$ "  $\phi$  x 8" (5" Min. Embed. Schemes 4 & 6)

Adhesive-Bonded Anchors with Heavy Hex Nuts and

Washers set in drilled holes  $(1'-1\frac{1}{2})''$  or  $5\frac{1}{2}''$  Max.

or "C" (See Roadway Plans)

Plumb (Max.) \*

Existing Wing Post

Existing Wing

(Type Varies)

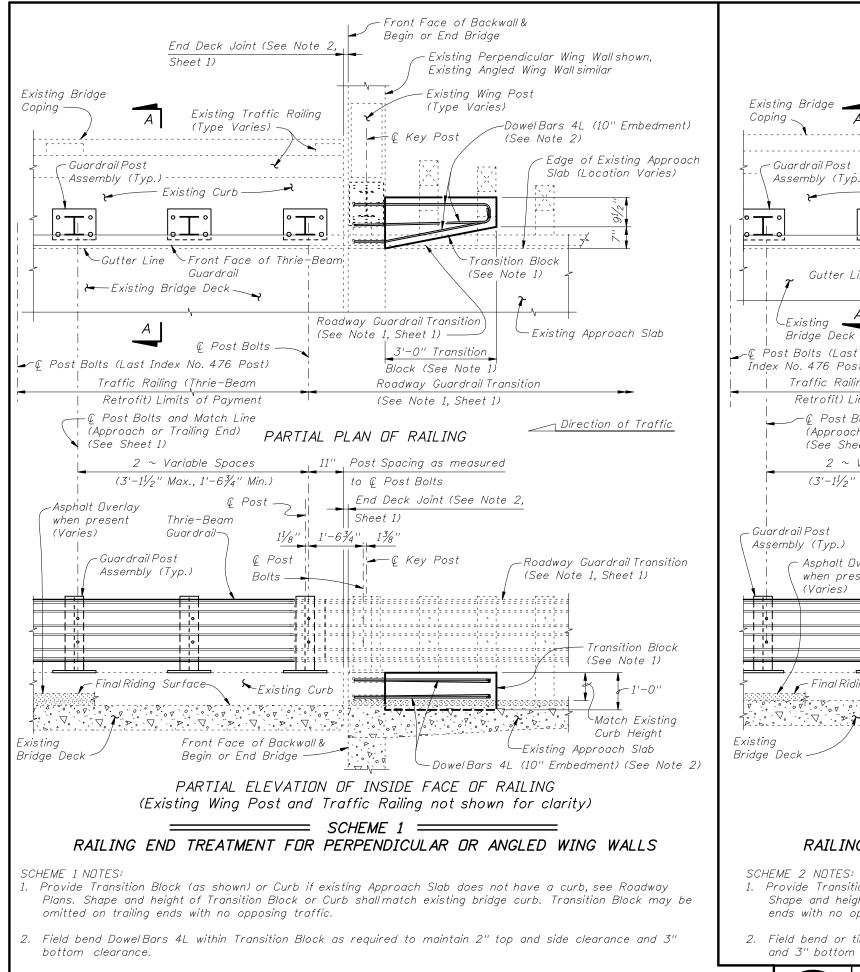
For location of Section C-C see Sheet 3.

For application of Dim. A see Post Dimension Table

on Index 470, Sheet 3.



#### 2010 FDOT Design Standards



RAILING END TREATMENT FOR PARALLEL OR FLARED CURBS WITH DETACHED SIDEWALKS OR INTEGRAL SIDEWALK LESS THAN 6" THICK SCHEME 2 NOTES: Shape and height of Transition Block or Curb shall match existing bridge curb. Transition Block may be omitted on trailing ends with no opposing traffic and on bridges with flared Approach Slab Curbs. 2. Field bend or tilt DowelBars 4D and Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance. 2010 FDOT Design Standards Sheet No. 01/01/08 3 of 4 TRAFFIC RAILING - (THRIE-BEAM RETROFIT) 476

**WIDE CURB TYPE 2** 

Existing Detached Sidewalk or Integral Sidewalk

Plans for Details of Sidewalk replacement.

Double Dowel Bars 4D (10"

Embedment) (See Note 2)-

Existing Approach

- Roadway Guardrail Transition

(See Note 1, Sheet 1)

Roadway Guardrail Transition (See Note 1, Sheet 1)

Existing Curb Double Dowel Bars 4D (10"

Embedment) (See Note 2)

Existing Approach Slab

Existing Flared Wing

Wall shown, Existing Parallel Wing Wall similar

Existing Wing Post

Post

(See Note 1, Sheet 1)

PARTIAL PLAN OF RAILING

−@ Key Post

11" Post Spacing as measured

to & Post Bolts

Sheet 1)

1'-63/4" 13/8"

Roadway Guardrail Transition

End Deck Joint (See Note 2,

(Type Varies)

less than 6" thick to be removed. See Roadway

Approach Slab Curb to

shown, Flared Curb similar)

Bars 4M

remain (Parallel Curb

2 sp. @ 1'-3"

.3'-0''

Transition Block

(See Note 1)

Edge of Existing Approach

Slab (Location Varies) -

→ Direction of Traffic

Transition Block

Curb Height) -

-Bars 4M

(See Note 1) Varies (Match

PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post and Traffic Railing not shown for clarity) \_\_\_\_\_\_ SCHEME 2 \_\_\_

Front Face of Backwall &

End Deck Joint (See

Note 2, Sheet 1)

Existing Traffic Railing

Front Face of Thrie-Beam

Begin or End Bridge

(Type Varies)

- Existina Curb

Guardrail

€ Post Bolts ~

@ Post -

@ Post

Front Face of Backwall &

Begin or End Bridge ——

Existina Bridge

- Guardrail Post

Existing

Index No. 476 Post)

- Gu'ardrail Post

Assembly (Typ.)

Bridge Deck

Assembly (Typ.)

Gutter Line

Traffic Railing (Thrie-Beam

(See Sheet 1)

Asphalt Overlay

when present

(Varies)

Retrofit) Limits of Payment

© Post Bolts and Match Line

(Approach or Trailing End)

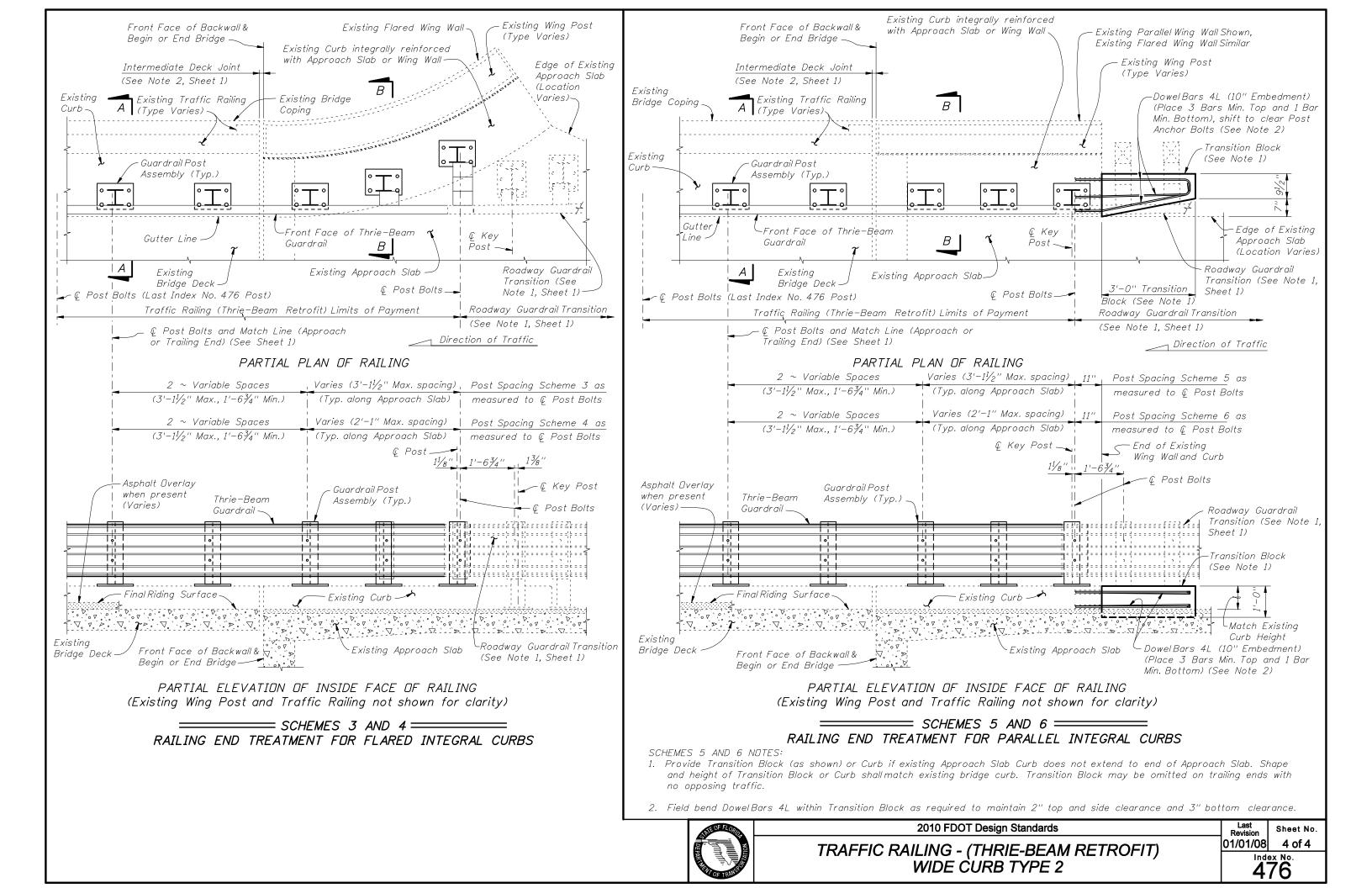
2 ~ Variable Spaces

 $(3'-1\frac{1}{2}'')$  Max.,  $1'-6\frac{3}{4}''$  Min.)

Thrie-Beam

Guardrail -

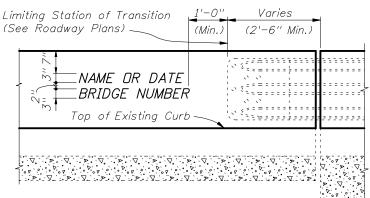
1. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend to end of Approach Slab.



#### = TRAFFIC RAILING NOTES =

- This Traffic Railing Retrofit has been structurally evaluated to be equivalent or greater in strength to a design which has been successfully crash tested previously and approved for a NCHRP Report 350 Test Level 4 rating, except for the Tapered End Transition on Index No. 484.
- CONCRETE: Concrete for the Traffic Railing (Vertical Face Retrofit), Spread Footing Approaches and replacement curb sections shall be Class IV. Concrete for Curb Transition Blocks shall be Class II (Bridge Deck).
- REINFORCING STEEL: Reinforcing steel shall be ASTM A615, Grade 60, except Expansion Dowel Bar B which shall be ASTM A36 smooth round bar hot-dip galvanized in accordance with the Specifications.
- EXPANSION SLEEVE ASSEMBLY: Pipe sleeve shall be ASTM D2241 PVC pipe, SDR13.5. End Cap shall be ASTM D2466 PVC socket fitting, Schedule 40. End of Sleeve assembly at railing open joint shall be sealed with silicone to prevent concrete intrusion during railing casting. A compressible expanded polystyrene plug is required in the opposite end of the assembly for correct dowel positioning during railing casting. Correct dowel positioning is required in order to provide for thermal movement of the deck.
- ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels shall comply with Specification Section 937 and be installed in accordance with Specification Section 416. The field testing proof loads required by Specification Section 416 shall be 23,800 lbs. for Dowel Bars 6D on the inside face (traffic side) of the railing (1'-0'' embedment) and 18,500 lbs for Dowel Bars 6D along the outside face of the traffic railing (5'' min. embedment).
- BRIDGES ON CURVED ALIGNMENTS: The details presented in these Standards are shown for bridges on tangent alignments.

  Details for bridges on horizontally curved alignments are similar.
- NAME, DATE AND BRIDGE NUMBER: The Name and Bridge Number shall be placed on the Traffic Railing so as to be seen on the driver's right side when approaching the bridge. The Date shall be placed on the driver's left side when approaching the bridge. The Date shall be the year the bridge was constructed. Letters and figures may be 3" tall black plastic as approved by the Engineer or 3/6" V-Grooves. V-Grooves shall be formed by preformed letters and figures.
- ELEVATION MARKERS: Elevation Markers shall be placed on the top surface of the end bents as directed by the Engineer when portions of the existing traffic railing carrying existing elevation markers are removed. Markers are to be furnished by the Florida Department of Transportation and installed by the Contractor.
- SURFACE FINISH: Unless otherwise shown in the Plans, place a Class 5 Applied Finish Coating on the top and sides of the Traffic Railing (Vertical Face Retrofit).
- REFLECTIVE RAILING MARKERS: Reflective Railing Markers shall meet Specification Section 993. Install markers on top of the Traffic Railing 2" from the face on the traffic side at the spacing shown in the table below. Reflector color (white or yellow) shall match the color of the near edgeline.
- PAYMENT: Payment under Traffic Railing (Vertical Face Retrofit) include all materials and labor required to construct the railing. Incidental work as required for transition blocks, curbs, spread footings approaches, reflective railing markers (including installation) shall also be included under Traffic Railing (Vertical Face Retofit).

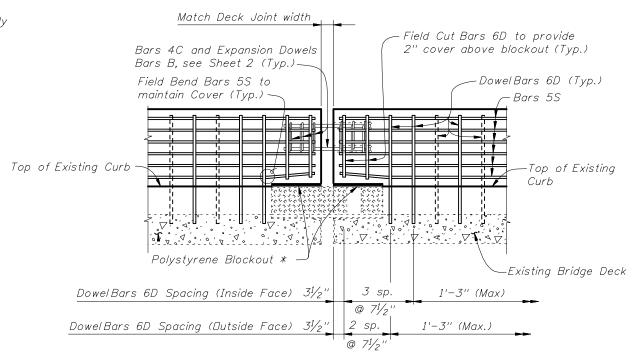


NAME,	DATE	AND	В	RIDGI	Ε	NUMBER
	LETT	ERIN	G	DETA	41	<u>/</u>

ESTIMATED TRAFFIC RAILING QUANTITIES								
LINIT	QUANTITY							
ONT	9" Curb	Increment						
CY/Ft.	0.064	0.003 per in. height						
Lb./Ft.	13.27	0.10 per in. length						
	UNIT	UNIT QUAN 9" Curb  CY/Ft. 0.064						

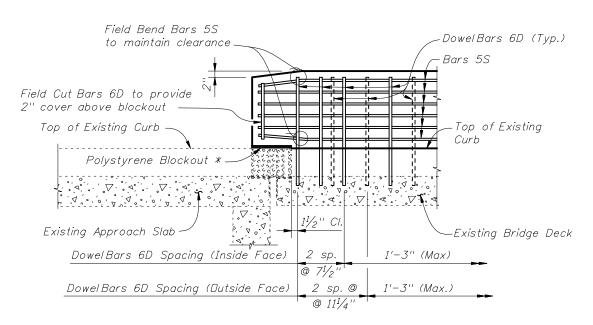
REFLECTIVE RAILING MARKER SPACING					
Distance — Edge of TravelLane to Face of Railing	Spacing (Ft.)				
< 4'	40'				
4' to 8'	80'				
> than 8'	None Required				

(Quantities are based on a 9" curb, no curb cross slope and 1'-0" embedment length of Bars 6D. If the curb height or embedment length differs from that shown, increase or decrease quantity by the given per inch increment.) See Index No. 484, Sheet 4 for Spread Footing Approach Quantities.



PARTIAL ELEVATION OF RAILING SHOWING INTERIOR FINGER/SLIDING PLATE JOINT (Beam/Girder, Intermediate Bent or Pier not shown for clairty)

\* Place 1" thick polystyrene blockout over limits of bridge deck expansion joint full width to the end of the Traffic Railing to allow for thermal movement. Seal Forms to prevent mortar leakage into the expansion joint.



PARTIAL ELEVATION OF RAILING SHOWING SLIDING PLATE JOINT AT BEGIN OR END BRIDGE (Scheme 1 shown, Schemes 2, 3 and 4 similar)

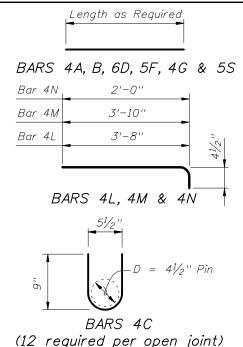
(Guardrail Transition or continuation of Traffic Railing not show for clarity)

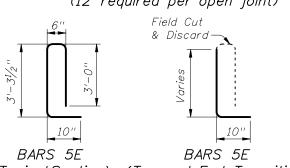


#### CONVENTIONAL REINFORCING STEEL BENDING DIAGRAM BILL OF REINFORCING STEEL MARK SIZE **LENGTH** INDEX NO. NOTE NOS. AS REQD. 482 ONLY 3 В 1" Ø 2'-0" 481 THRU 483 2 & 5 C2'-0" 481 THRU 484 1, 2 & 3 D 2 & 3 AS REQD. 481 THRU 484 Ε 5 7'-4" 1 & 3 484 ONLY 5 4'-3" 3 484 ONLY G4 AS REQD. 484 ONLY 3 4 4'-1" 481 THRU 483 1 & 3 Μ 4'-3" 1 & 3 482 ONLY Ν 2'-5" 482 ONLY 1 & 3 4 S AS REQD 481 THRU 484 2, 3 & 4

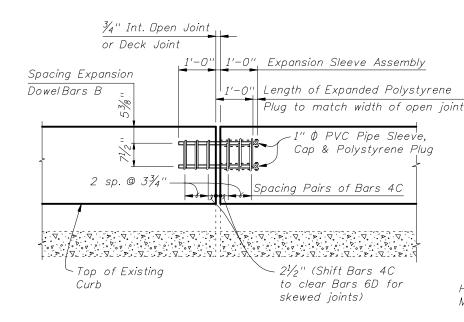
#### REINFORCING STEEL NOTES:

- 1. All bar dimensions in the bending diagrams are out to out. 2. The reinforcement for the railing on a retaining wall shall be
- the same as detailed for a bridge deck.
- 3. All reinforcing steel in the Vertical Face Retrofit Railing shall have a 2" minimum cover.
- 4. Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-0".
- 5. Expansion Dowel Bars B shall be ASTM A36 smooth round bar and hot-dip galvanized in accordance with the Specifications.

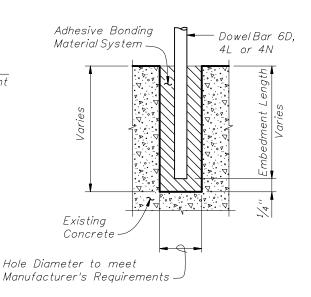




(Typical Section) (Tapered End Transition)



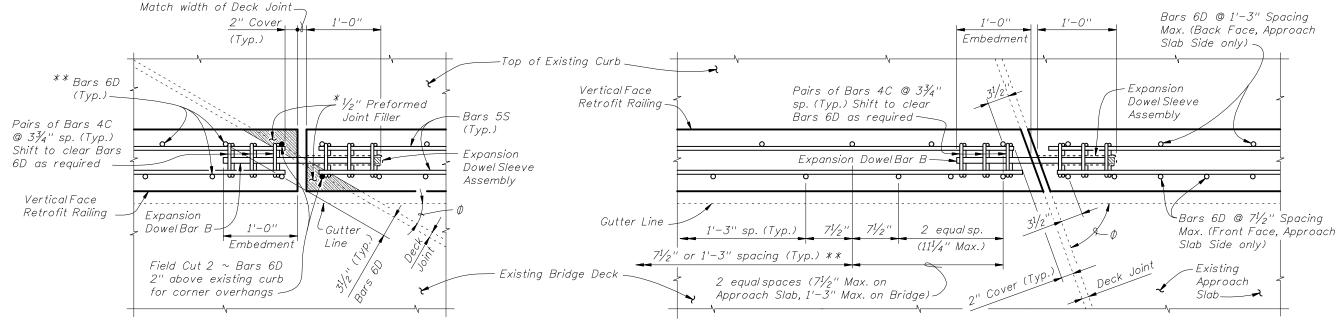
OPEN JOINT EXPANSION DOWEL DETAIL (Railing Reinforcing Not Shown For Clarity)



#### DOWEL DETAIL

Dowel Installation Notes:

- 1. Shift dowel holes to clear if the existing reinforcement is encountered.
- 2. See individual Standards Index Nos. 481 thru 484 for required embedment length of Bars 6D, 4L or 4N.
- $\frac{1}{2}$ " Preformed Joint Filler at top of Existing Curb shall extend beyond the joint material (Silicone, poured rubber, armored neoprene seal or sliding plates) as shown to prevent concrete intrusion during railing casting and shall be placed so as not to restrict in any way normal joint movement.
- \*\* See individual Standard Index Nos. 481 thru 484 for spacing of Bars 6D.



PARTIAL PLAN OF RAILING (SKEW ANGLE Ø LESS THAN 70°) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

PARTIAL PLAN OF RAILING (SKEW ANGLE  $\phi = 70^{\circ}$  OR GREATER) (Skewed Deck Joint at Begin or End Bridge Shown, Skewed Deck Joint at Intermediate Pier or Bent Similar)

SKEW DETAIL



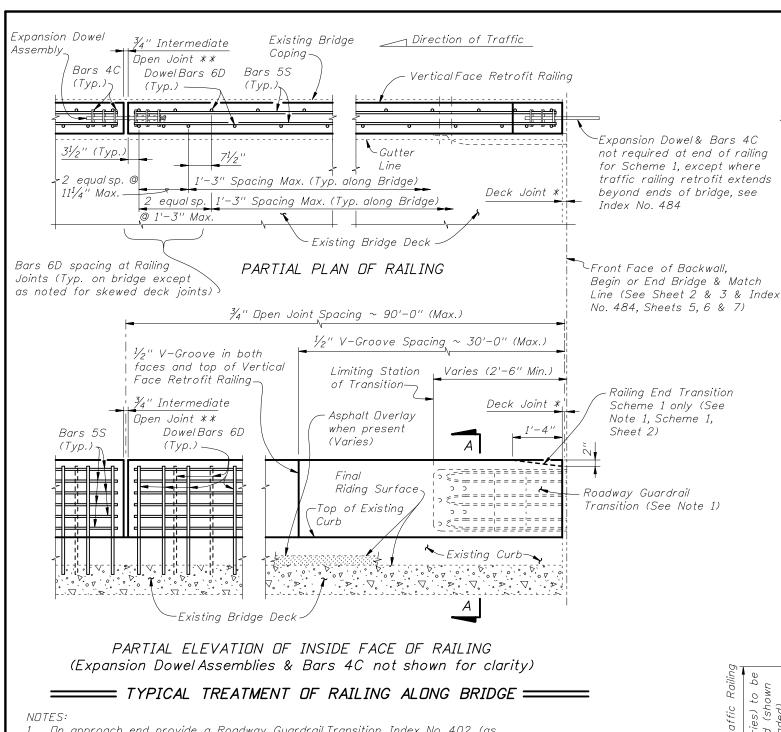
2010 FDOT Design Standards

TRAFFIC RAILING - (VERTICAL FACE RETROFIT) GENERAL NOTES & DETAILS

07/01/09 2 of 2

Sheet No.

1ndex No. 480

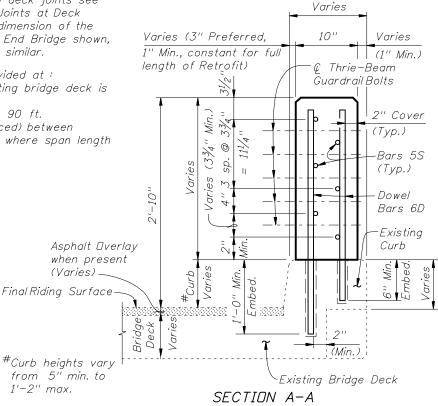


- 1. On approach end provide a Roadway Guardrail Transition, Index No. 402 (as shown) or other site specific treatment. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is on the bridge, attach Thrie Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is along the Wing Wall, see Schemes 2 or 3, Index No. 481, Sheet 2 and 3. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing. For treatment of trailing end see Roadway Plans. If vertical face retrofit extends beyond bridge and approach slab ends, see Index No. 484 for treatment and Details.
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.
- 3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off 1" below existing concrete and grouted over.

Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index No. 480. Open Railing Joints at Deck Expansion Joint locations shall match the dimension of the Deck Joint. Deck Joint at Begin Bridge or End Bridge shown, Deck Joint at & Pier or Intermediate Bent similar.

\*\* $^3\!\!\!/_4$ " Intermediate Open Joints shall be provided at :

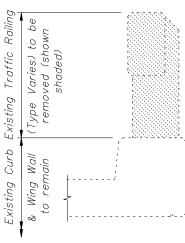
- (1) Substructure supports where existing bridge deck is continuous.
- (2) Midspan where span length exceeds 90 ft.
- (3) Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.



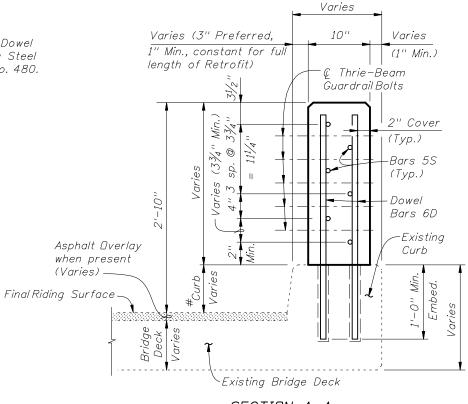
SECTION A-A
TYPICAL SECTION THRU RAILING
ON CURB WITH CORBELS

#### CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagrams see Index No. 480.



TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)



SECTION A-A TYPICAL SECTION THRU RAILING ON FULL DEPTH CURB (BRIDGE SHOWN, WING WALL SIMILAR)

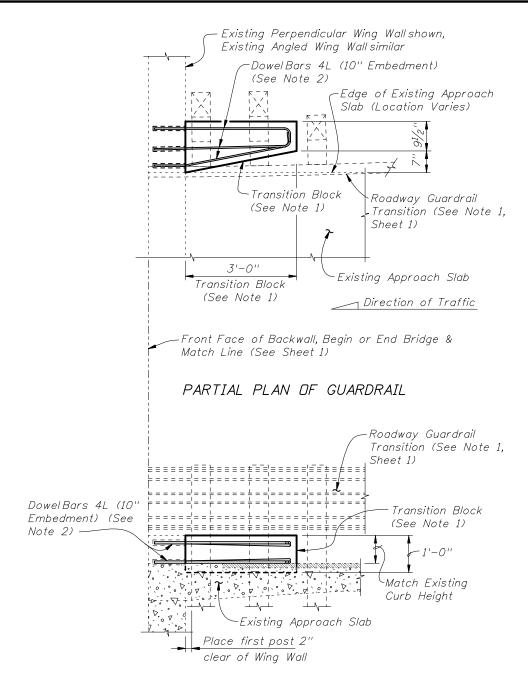


2010 FDOT Design Standards

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
NARROW CURB

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Index No. 481

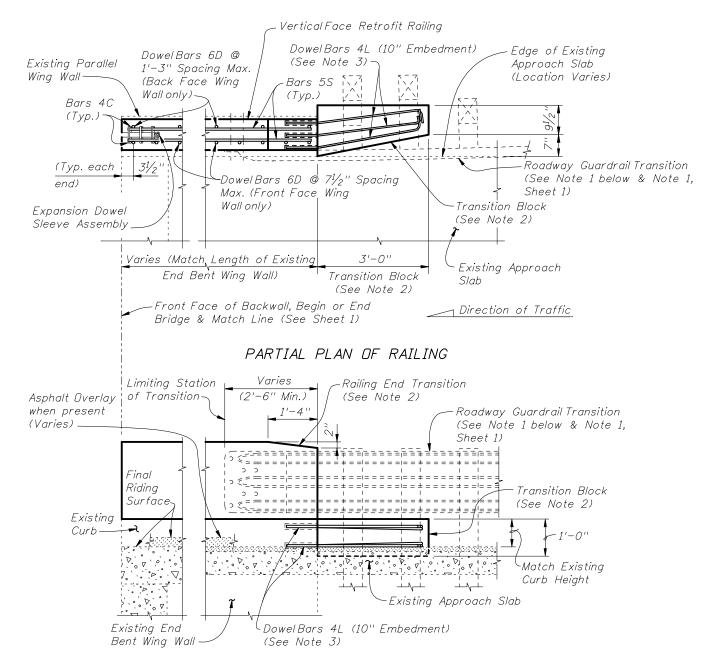


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

## RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

#### SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



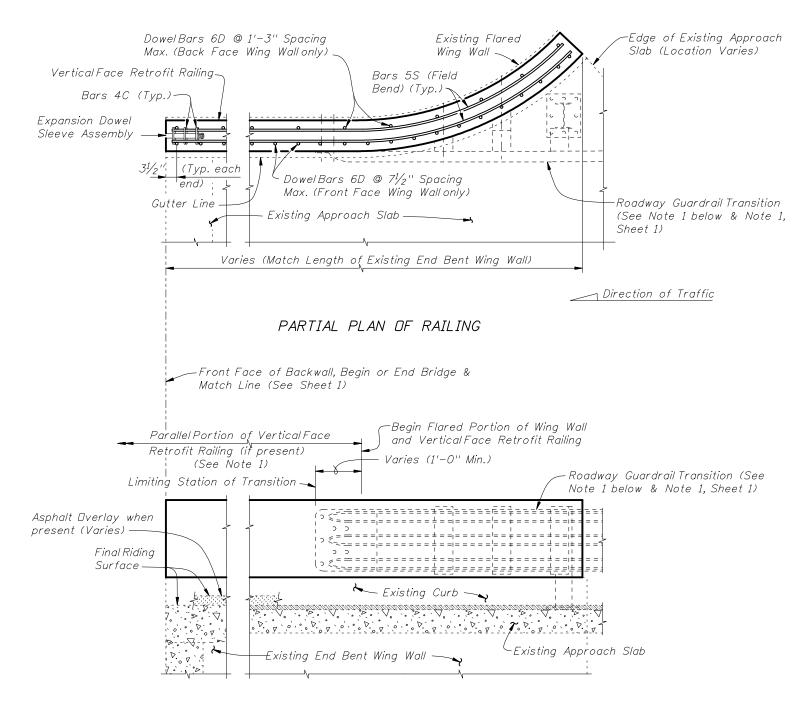
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

## RAILING END TREATMENT FOR PARALLEL WING WALLS

SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Index No. 481, Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



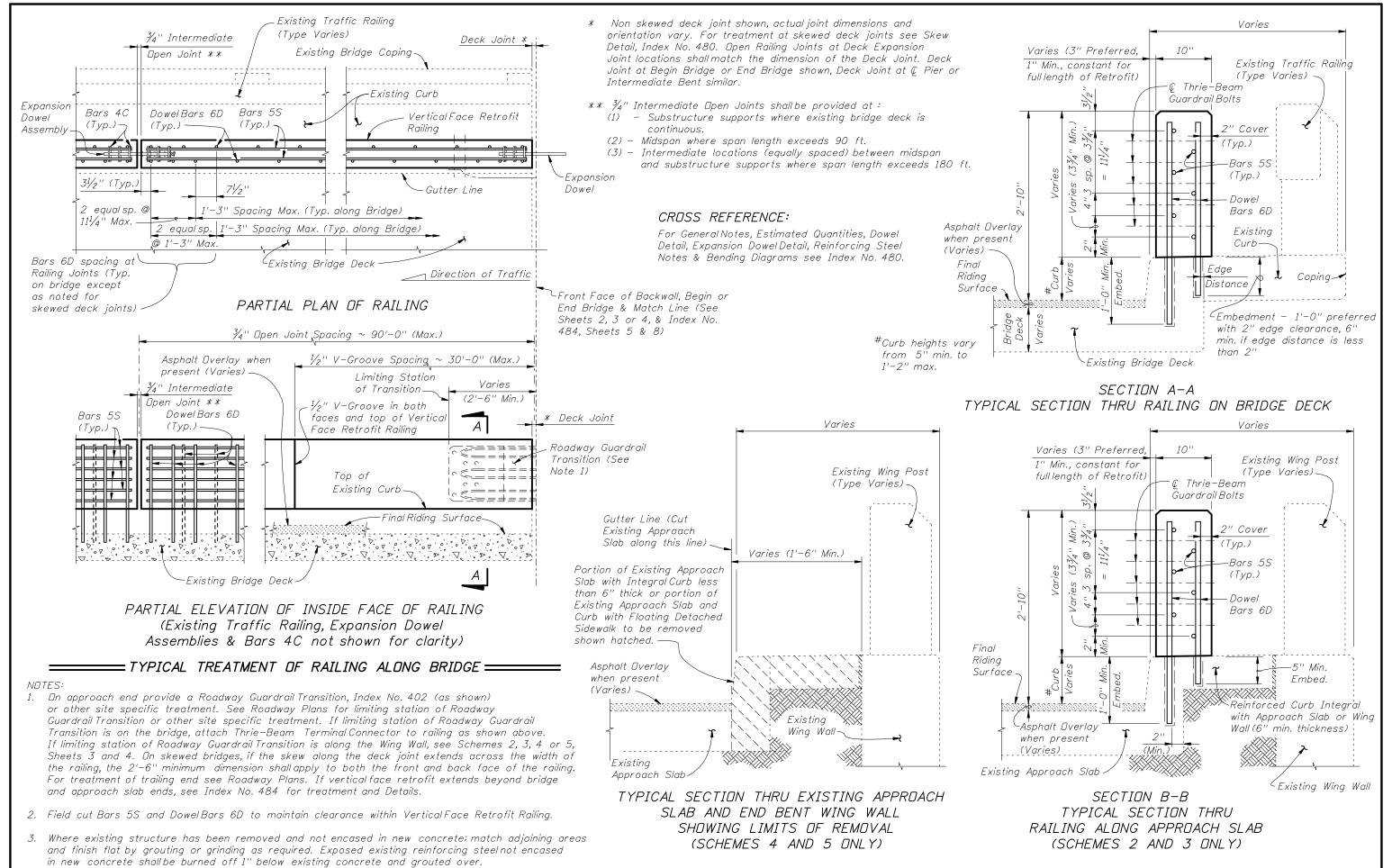


PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

#### SCHEME 3 NOTE:

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.

Index No.



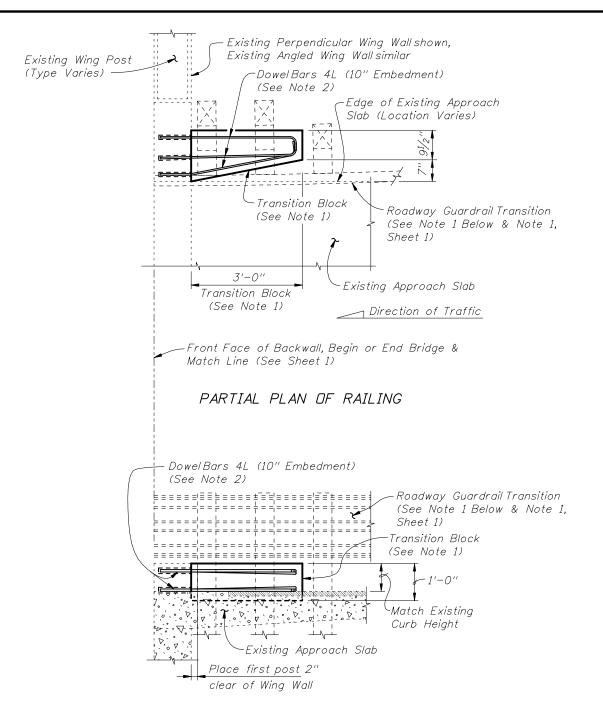


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TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
WIDE CURB

Last Sheet No. 07/01/09 1 of 4

1ndex No. 482

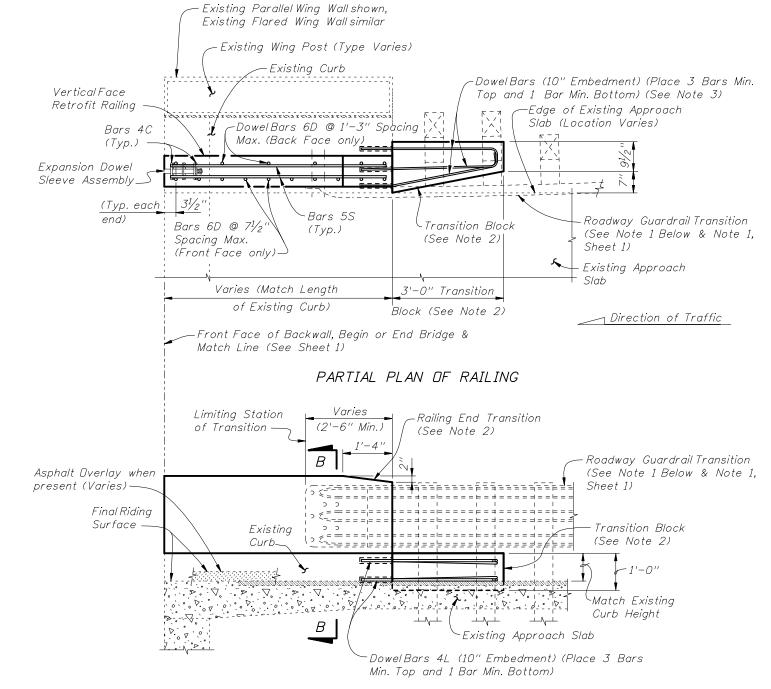


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL (Existing Wing Post not shown for clarity)

### RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

#### SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



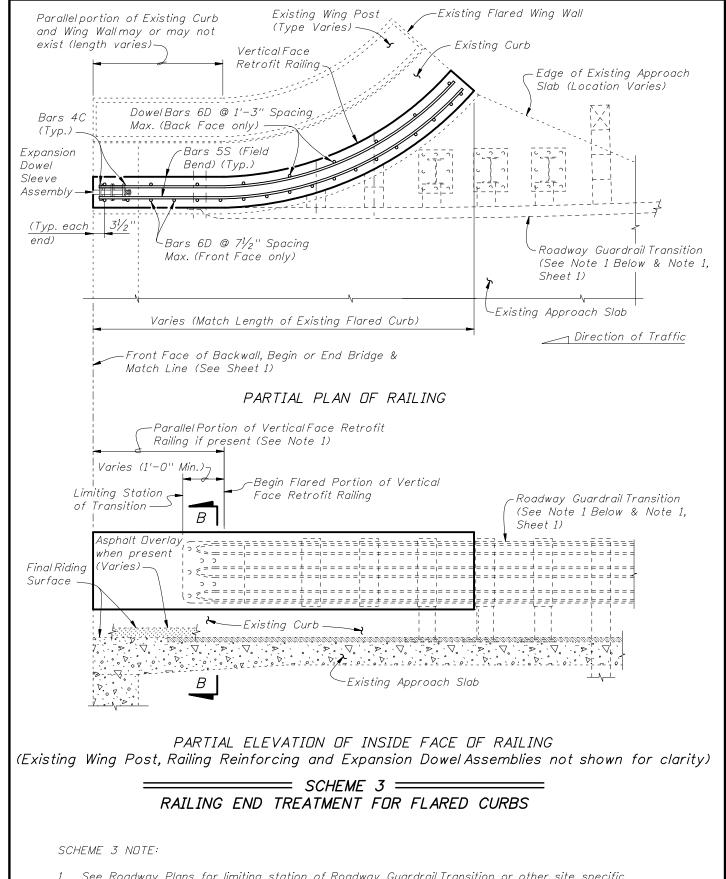
PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

### RAILING END TREATMENT FOR PARALLEL CURBS

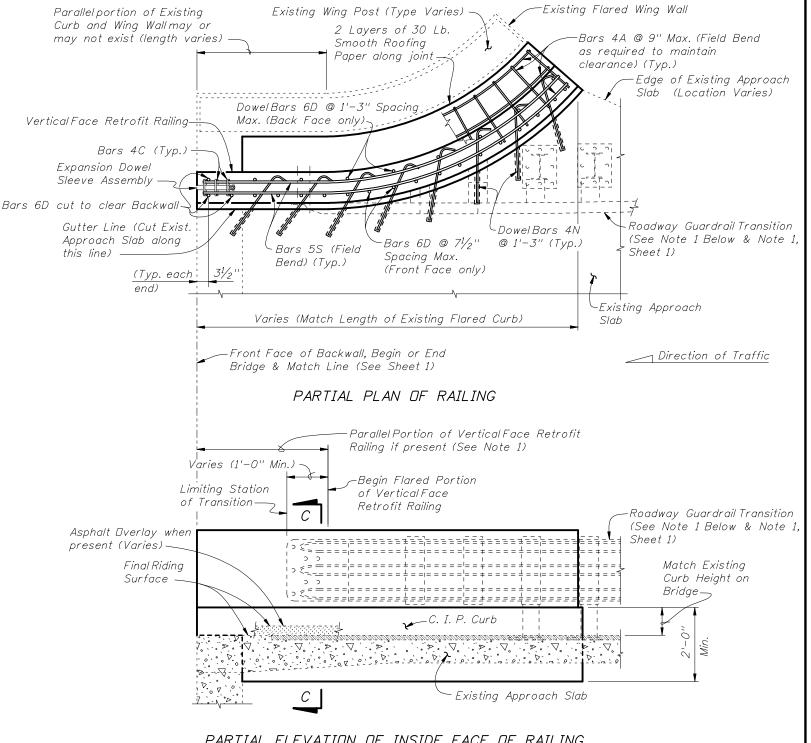
SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shallmatch existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend Dowel Bars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.





1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

SCHEME 4 NOTES:

RAILING END TREATMENT FOR FLARED CURBS

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see , Sheet 1.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.



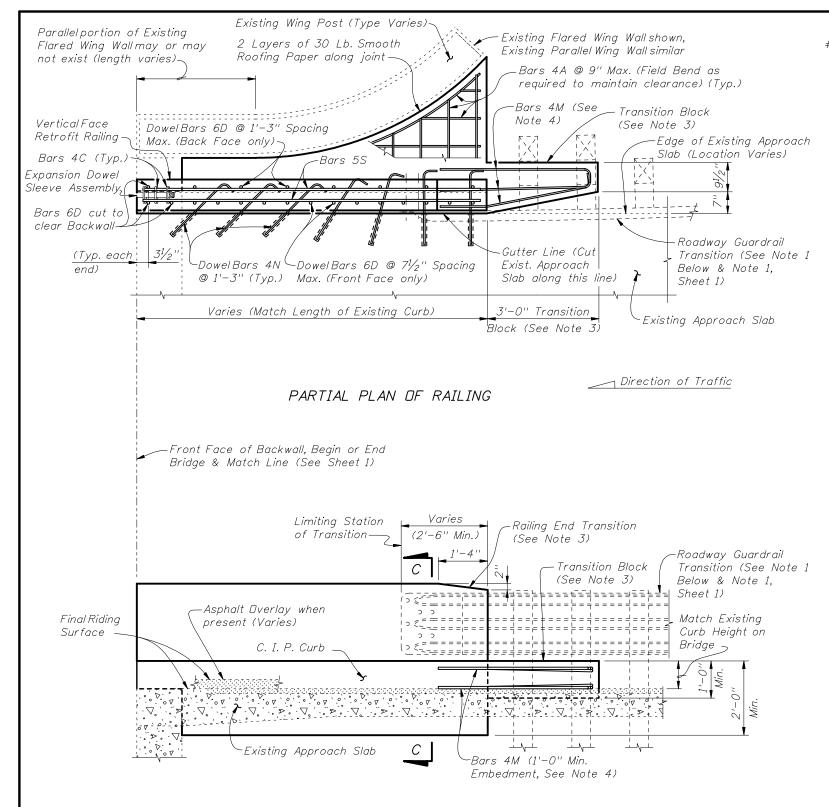
2010 FDOT Design Standards

**WIDE CURB** 

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)

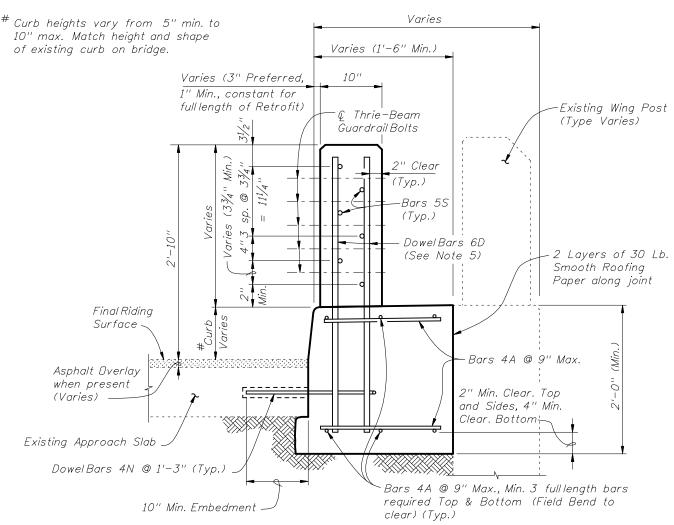
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1ndex No. 482



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

RAILING END TREATMENT FOR PARALLEL CURBS

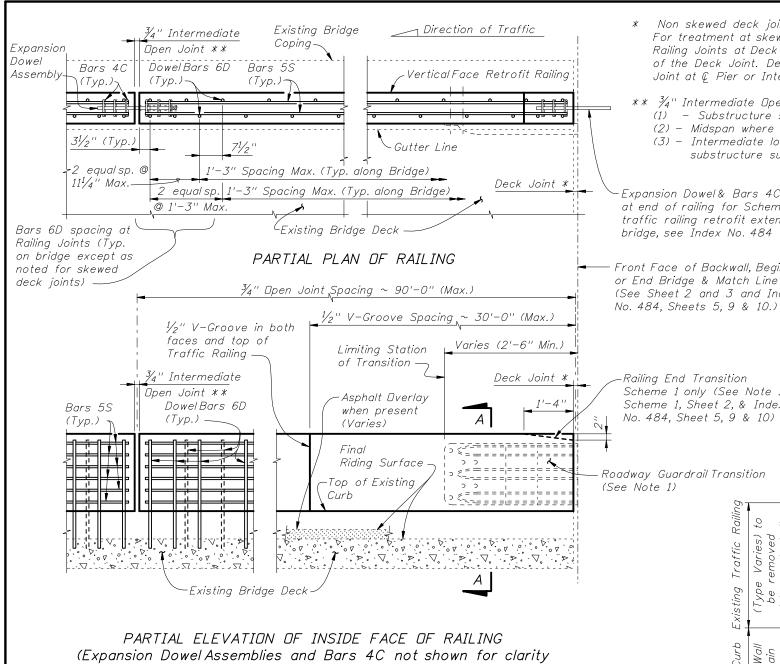


SECTION C-C TYPICAL SECTION THRU RAILING ALONG APPROACH SLAB (SCHEME 4 SHOWN, SCHEME 5 SIMILAR)

SCHEME 5 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.
- 2. Dowel Bars 4N may be installed on a maximum angle of 45° to the cut edge of the Approach Slab as shown to facilitate drilling of holes and installation of bars.
- 3. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 4. Field bend Dowel Bars 4M within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 5. At the Contractor's option, along the length of the Approach Slab curb that is to be replaced, Dowel Bars 6D may be cast in with the new section of curb as shown or they may be installed in drilled holes in the new section of curb using an Adhesive Bonding Material System with a 1'-0" minimum embedment.





#### == TYPICAL TREATMENT OF RAILING ALONG BRIDGE =

#### NOTES:

- 1. On approach end provide a Roadway Guardrail Transition, Index No. 402 (as shown) or other site specific treatment. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is on the bridge, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is along the Wing Wall, see Schemes 2 or 3, Sheets 2 & 3. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" minimum dimension shall apply to both the front and back face of the railing. For treatment of trailing end see Roadway Plans. If vertical face retrofit extends beyond bridge and approach slab ends, see Index No. 484 for treatment and Details.
- 2. Field cut Bars 5S and Dowel Bars 6D to maintain clearance within Vertical Face Retrofit Railing.
- 3. Where existing structure has been removed and not encased in new concrete; match adjoining areas and finish flat by grouting or grinding as required. Exposed existing reinforcing steel not encased in new concrete shall be burned off I" below existing concrete and grouted over.

\* Non skewed deck joint shown, actual joint dimensions and orientation vary. For treatment at skewed deck joints see Skew Detail, Index No. 480. Open Railing Joints at Deck Expansion Joint locations shall match the dimension of the Deck Joint. Deck Joint at Begin Bridge or End Bridge shown, Deck Joint at ¢ Pier or Intermediate Bent similar.

\*\*  $\frac{3}{4}$ " Intermediate Open Joints shall be provided at:

(1) - Substructure supports where existing bridge deck is continuous.

(2) - Midspan where span length exceeds 90 ft.

(3) - Intermediate locations (equally spaced) between midspan and substructure supports where span length exceeds 180 ft.

Expansion Dowel & Bars 4C not required at end of railing for Scheme 1, except where traffic railing retrofit extends beyond ends of

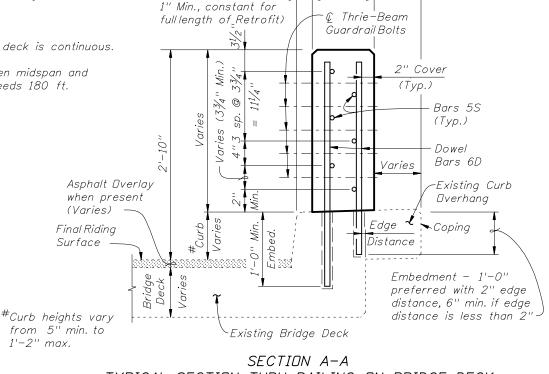
Front Face of Backwall, Begin (See Sheet 2 and 3 and Index

Scheme 1 only (See Note 1, Scheme 1, Sheet 2, & Index No. 484, Sheet 5, 9 & 10)

TYPICAL SECTION THRU EXISTING TRAFFIC RAILING SHOWING LIMITS OF REMOVAL (BRIDGE DECK SHOWN, WING WALL SIMILAR)

#### CROSS REFERENCE:

For General Notes, Estimated Quantities, Dowel Detail, Expansion Dowel Detail, Reinforcing Steel Notes & Bending Diagram see Index No. 480.

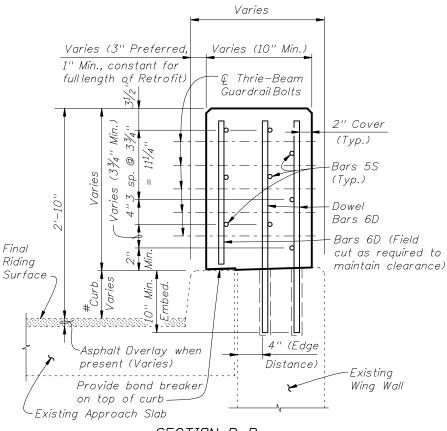


Varies (3" Preferred,

Varies

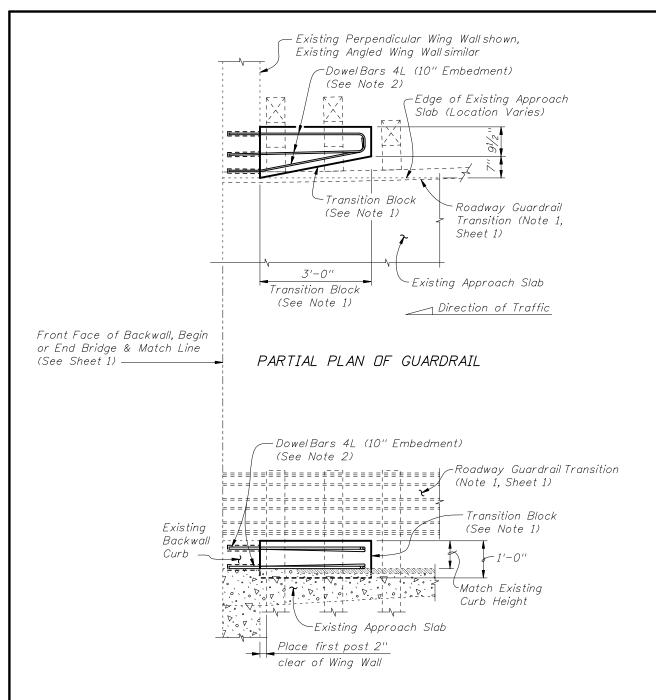
10"

TYPICAL SECTION THRU RAILING ON BRIDGE DECK



SECTION B-B TYPICAL SECTION THRU RAILING ON WING WALL



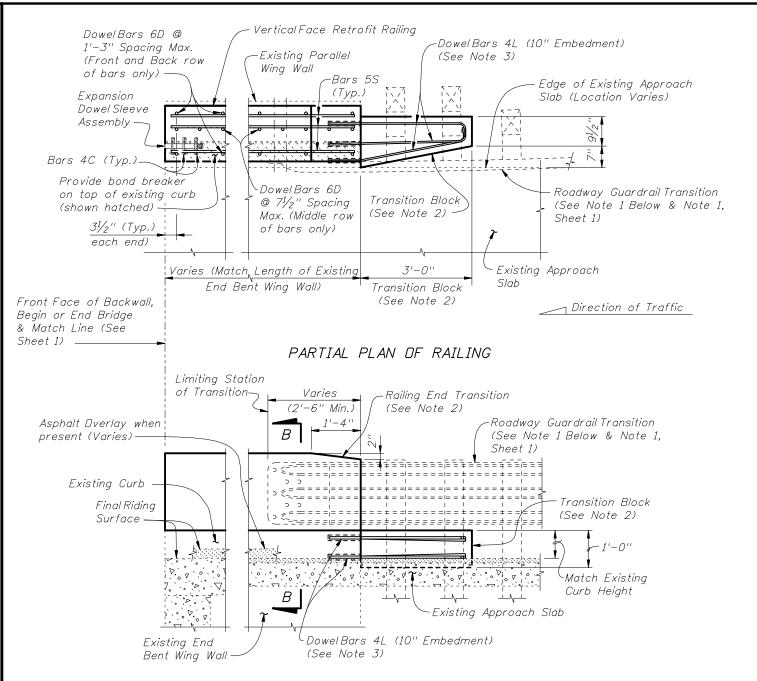


PARTIAL ELEVATION OF INSIDE FACE OF GUARDRAIL

## RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING WALLS

#### SCHEME 1 NOTES:

- 1. Provide Transition Block (as shown) or Curb if existing Approach Slab does not have a curb, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 2. Field bend DowelBars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.
- 3. If a Special Steel Guardrail Post is required for attachment to the top of a sloping Wing Wall, saw cut and remove a wedge shaped portion of the sloping Wing Wall as required to provide a level surface for post installation.



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

### RAILING END TREATMENT FOR PARALLEL WING WALLS

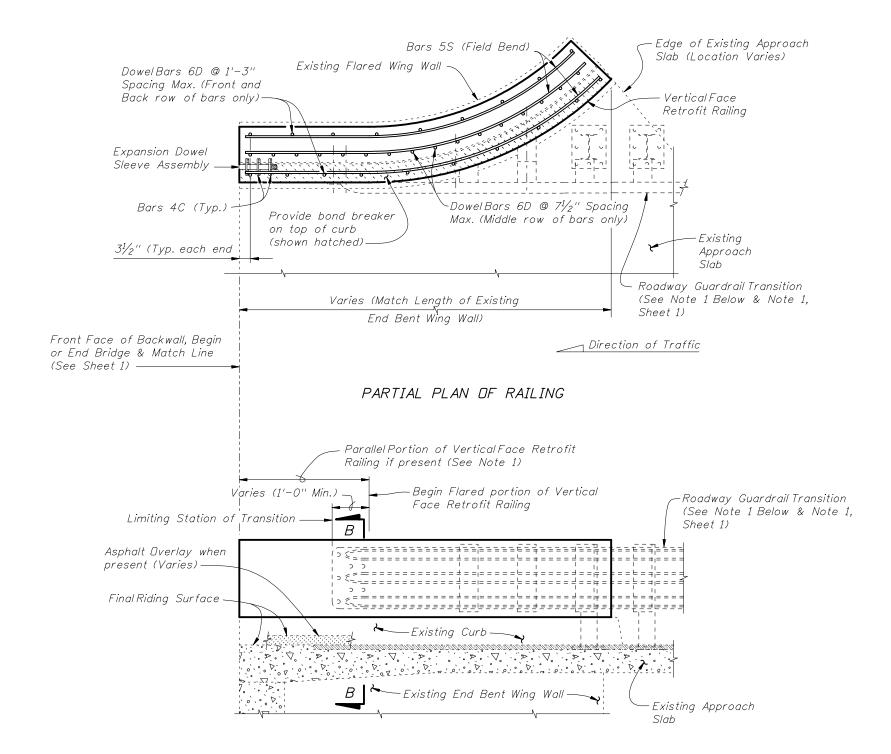
SCHEME 2 NOTES:

- 1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1. On skewed bridges, if the skew along the deck joint extends across the width of the railing, the 2'-6" mminimum dimension shall apply to both the front and back face of the railing.
- 2. Provide Transition Block (as shown) or Curb if existing Approach Slab Curb does not extend beyond end of existing End Bent Wing Wall, see Roadway Plans. Shape and height of Transition Block or Curb shall match existing bridge curb. Railing End Transition and Transition Block may be omitted on trailing ends with no opposing traffic.
- 3. Field bend DowelBars 4L within Transition Block as required to maintain 2" top and side clearance and 3" bottom clearance.



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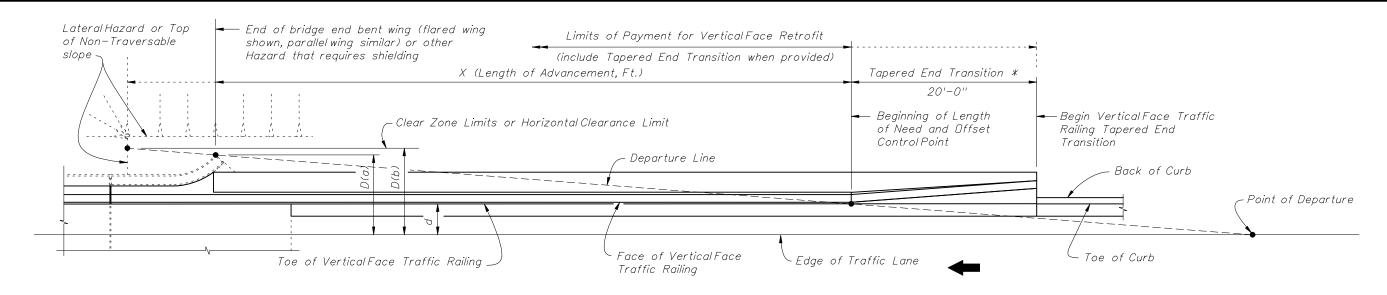


PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Railing Reinforcing and Expansion Dowel Assemblies not shown for clarity)

RAILING END TREATMENT FOR
FLARED WING WALLS

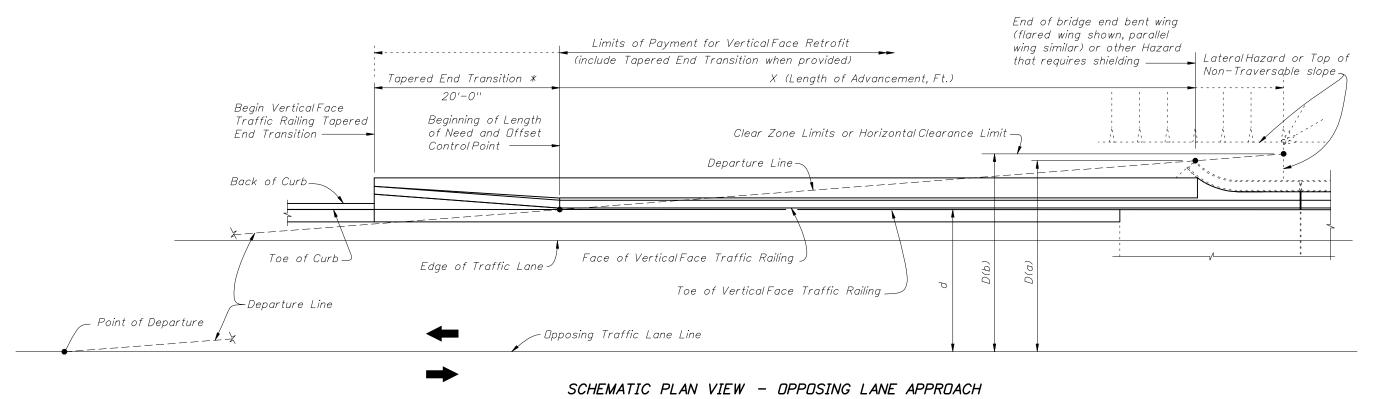
#### SCHEME 3 NOTE:

1. See Roadway Plans for limiting station of Roadway Guardrail Transition or other site specific treatment. If limiting station of Roadway Guardrail Transition is along the Wing Wall, attach Thrie-Beam Terminal Connector to railing as shown above. If limiting station of Roadway Guardrail Transition is on the bridge, see Sheet 1.



\* Guardrail or Crash Cushion may also be shown in the Contract Plans, in lieu of the Tapered End Transition.

#### SCHEMATIC PLAN VIEW - NEAR LANE APPROACH



# Design Speed (mph) Length of Advancement, Ft. (X) $\leq 40$ = 16 (D-d)

Notes

- 1. The minimum length of advancement for both near lane and opposing lane approaches is 20'.
- 2. For Design Speeds greater than 40 mph the Tapered End Transition is not permitted. See Index No. 400 for length of Advancement of guardrail or other project specific end treatments

#### ENOTE OF ADVANCEMENT TAREFER FUR TRANSITION (40 MBH GR LE

#### ===== LENGTH OF ADVANCEMENT - TAPERED END TRANSITION (40 MPH OR LESS) ======

DESIGN NOTES:

The Tapered End Transition should only be used when space is limited which precludes the use of a guardrail end treatment or crash cushion.

- D = Distance in feet from near edge of near approach traffic lane to either:
  - (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance;
  - (b) the clear zone or horizontal clearance outer limits, when hazard extends to, or goes beyond the clear zone or horizontal clearance limits.

For left side hazards on two way undivided facilities, "D" is measured from the inside edge of the near approach traffic lane as shown above.

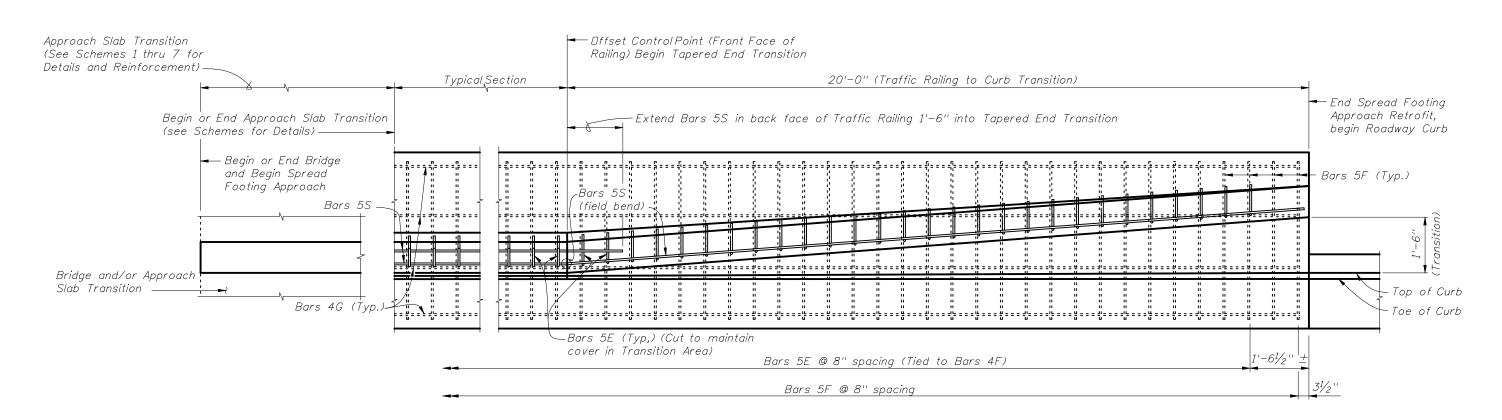
d = Distance in feet from near edge of near approach traffic lane to face of traffic railing (at offset control point). For left side hazards on two-way undivided facilities "d" is measured from the inside edge of the nearest opposing traffic lane as shown above.

#### CROSS REFERENCES:

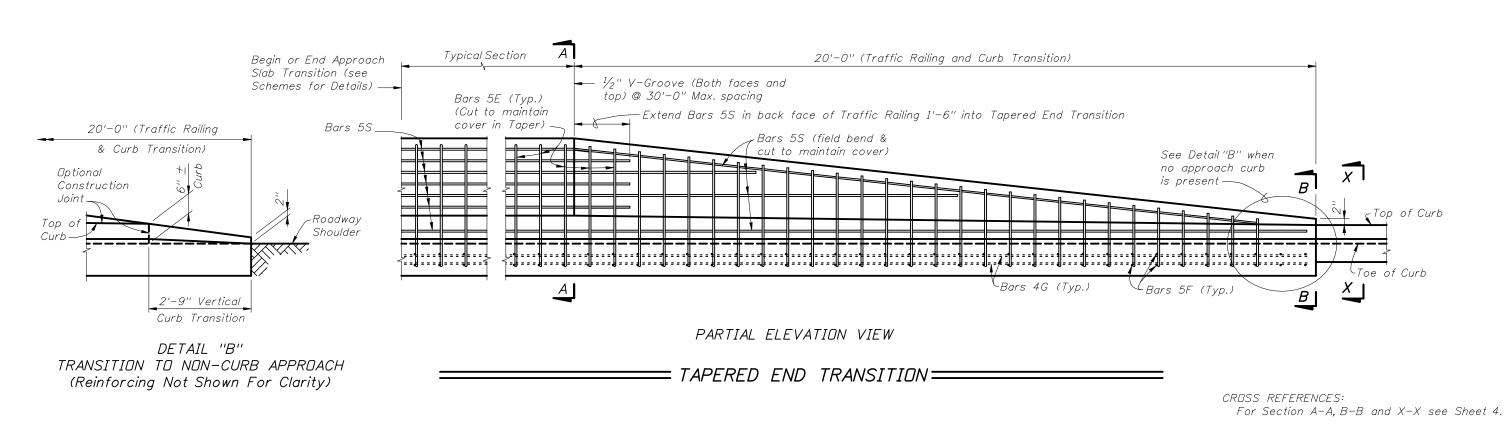
For General Notes, Dowel Details, Expansion Dowel Details, Reinforcing Steel Notes and Reinforcing Steel Bending Diagram see Index No. 480.



Sheet No.



#### PARTIAL PLAN VIEW

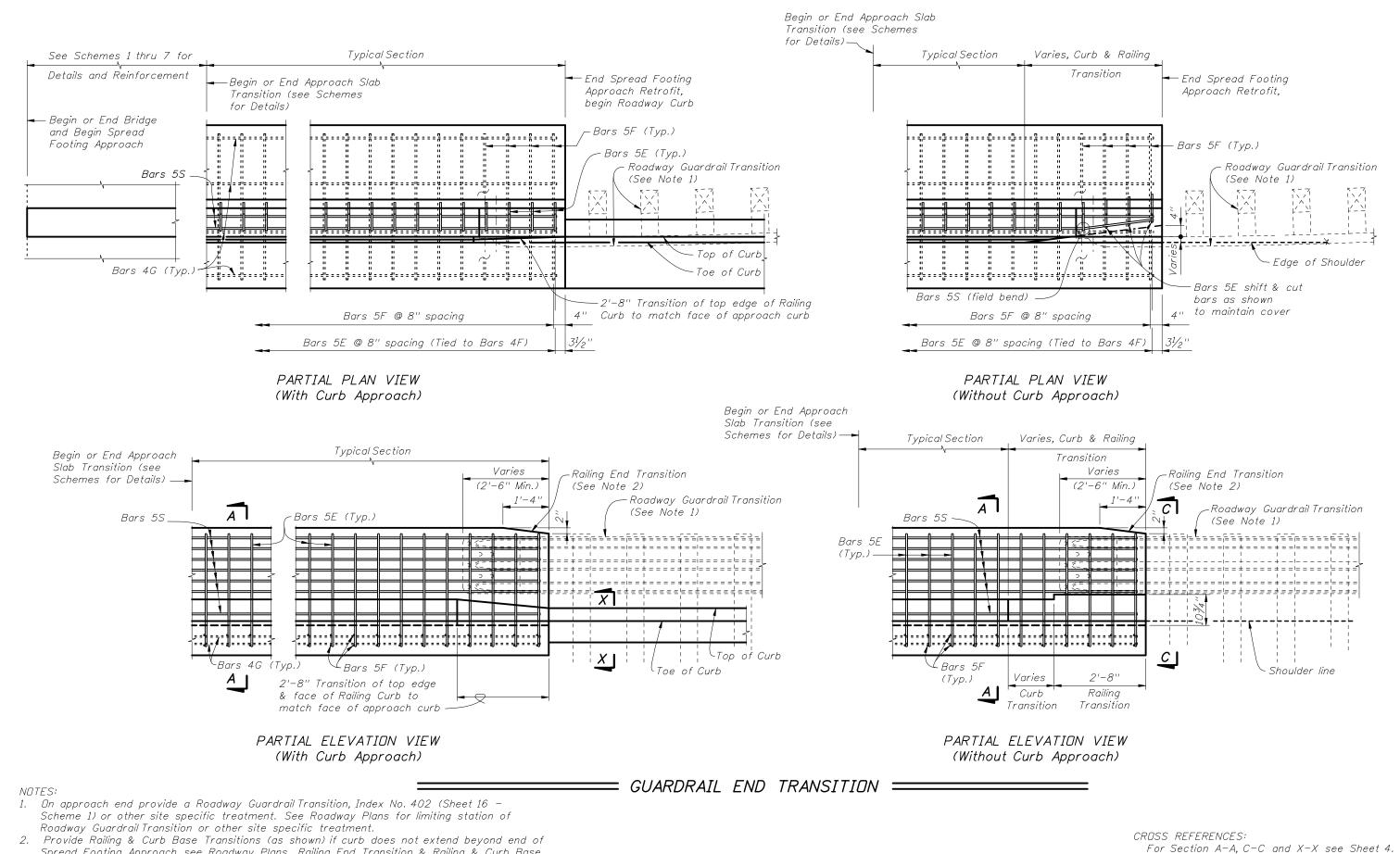




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TRAFFIC RAILING - (VERTICAL FACE RETROFIT) SPREAD FOOTING APPROACH



Spread Footing Approach, see Roadway Plans. Railing End Transition & Railing & Curb Base Transitions may be omitted on trailing ends with no opposing traffic.

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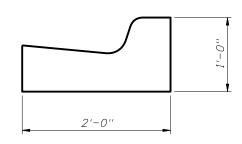
TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
SPREAD FOOTING APPROACH

Last Revision Sheet No. 07/01/09 3 of 10

Index No. 484

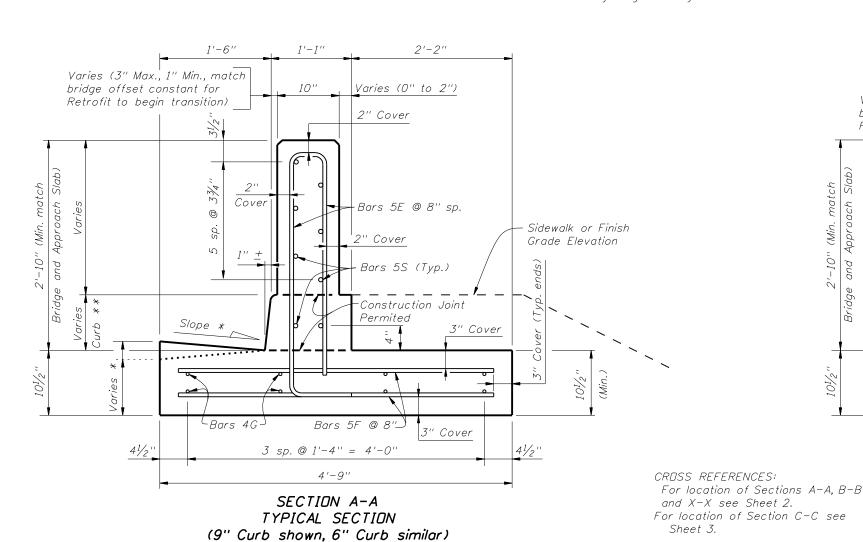
ESTIMATED TRAFFIC RAILING RETROFIT SPREAD FOOTING APPROACH QUANTITIES						
ITCM	UNIT	QUANTITY				
ITEM	UNIT	9" Curb				
Concrete - Typical Section	CY/Ft.	0.25				
Reinforcing Steel - Typical Section	Lb./Ft.	38				
Concrete - 20'-0" Tappered End Transition plus Footing	CY	4.57 Total				
Reinforcing Steel - 20'-0" Tapered End Transition plus Footing	Lb.	776 Total				

NDTE: Quantities are based on a 9" curb, no curb cross slope.



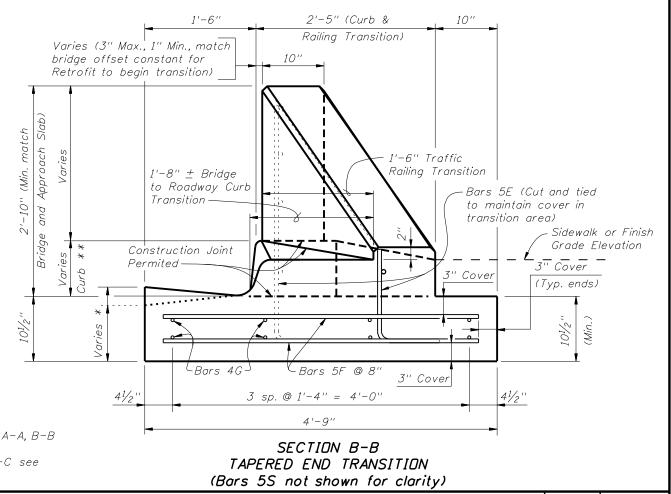
SECTION X-X (TYPICAL CURB, TYPE VARIES, TYPE F SHOWN) (See Index No. 300 and Plans for Details)

- \* Match Cross Slope of high side and low side at begin or end bridge or approach slab.
- \*\* Match curb height of adjacent bridge and approach slab. Adjust height in Transition area to match adjoining Roadway curb.



2'-2" 1'-6" Varies (3" Max., 1" Min., match bridge offset constant for Retrofit Varies (0" to 2") to begin Curb & Railing Transition) End Bar 5E (field cut & shift to maintain cover) Sidewalk or Finish Grade Elevation -Bars 5S (Typ.) Curb & Railing Construction Joint Transition-Permited Slope \* LBars 4G Bars 5F @ 8"\_ 3" Cover 3 sp. @ 1'-4'' = 4'-0''4'-9''

#### SECTION C-C (GUARDRAIL END TRANSITION)



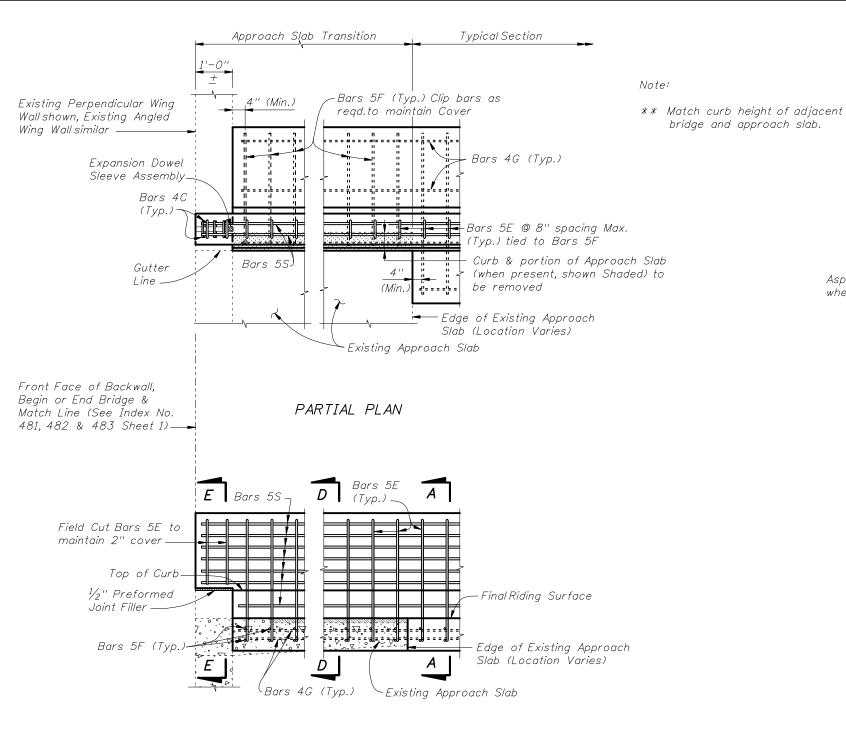


2010 FDOT Design Standards

TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
SPREAD FOOTING APPROACH

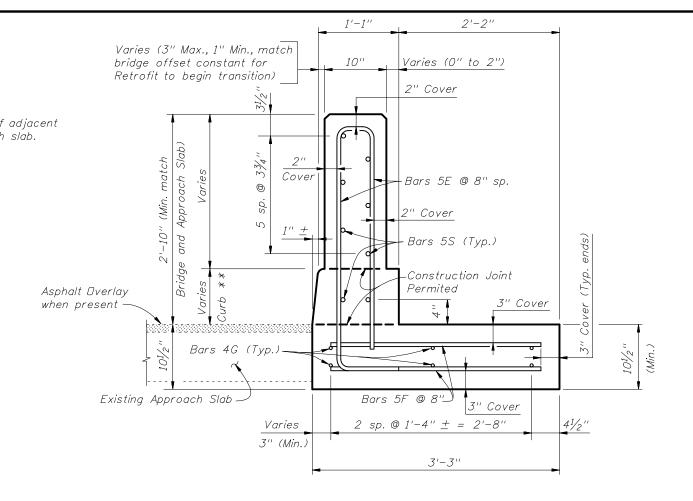
Last Sheet No. 07/01/09 4 of 10

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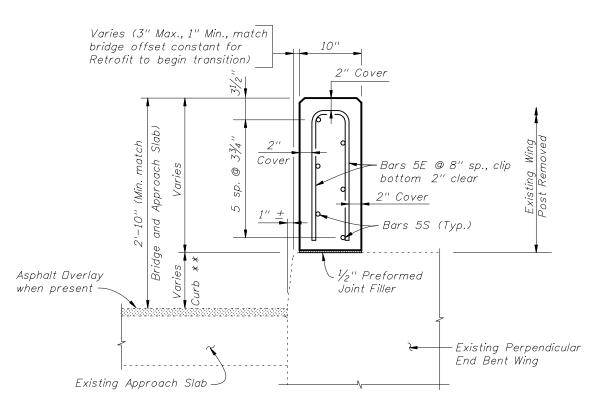


===SCHEME 1 ~ MODIFICATION FOR INDEX NO. 481, 482 AND 483 - SCHEME 1===
RAILING END TREATMENT FOR PERPENDICULAR OR ANGLED WING
WALLS WITH NARROW CURBS (SHOWN), WIDE CURBS
AND INTERMEDIATE CURBS (SIMILAR)

CROSS REFERENCE:
For Section A-A see Sheet 4.
For Expansion Dowel Assembly and
placement of Dowel Bars 6D Details
see Index 480.



#### SECTION D-D



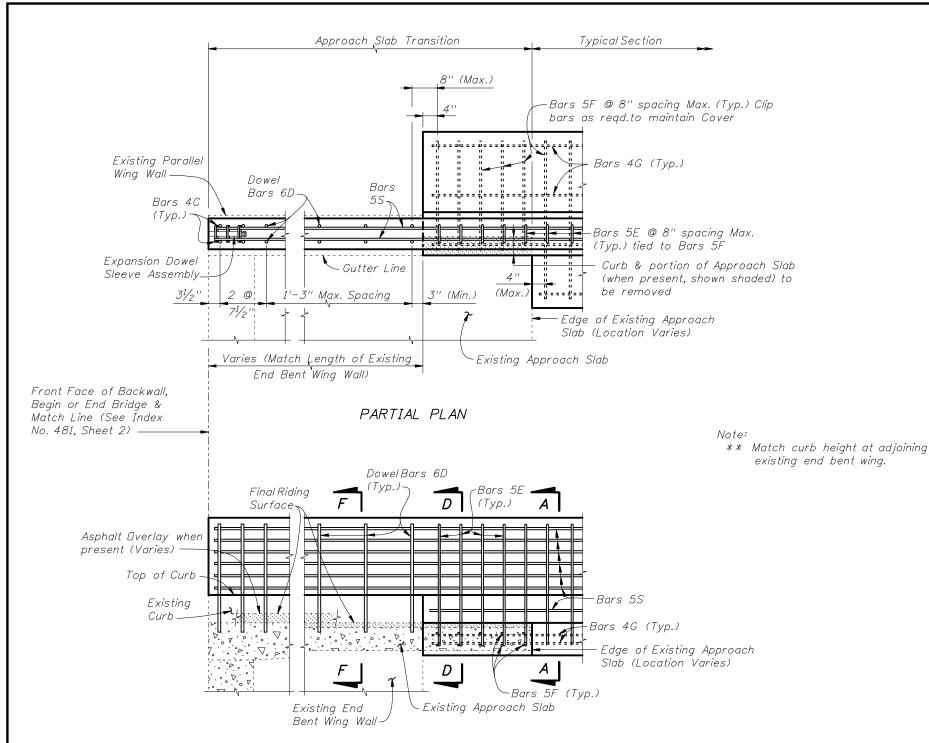
SECTION E-E (NARROW CURB SHOWN, WIDE AND INTERMEDIATE CURBS SIMILAR)



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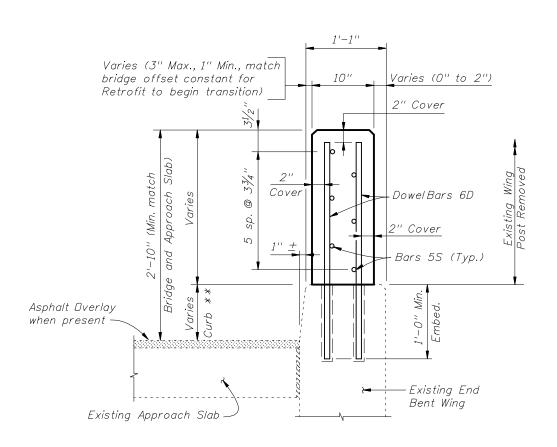
TRAFFIC RAILING - (VERTICAL FACE RETROFIT)
SPREAD FOOTING APPROACH

Index No. 484

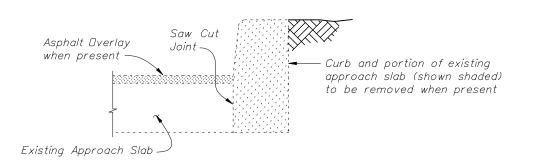


#### NOTES:

1. Remove existing concrete along saw cut joints. Existing reinforcing steel may be cut at joint or extended into new concrete. Exposed existing reinforcing not encased in new concrete shall be removed 1" below existing concrete surface and grouted over.



SECTION F-F



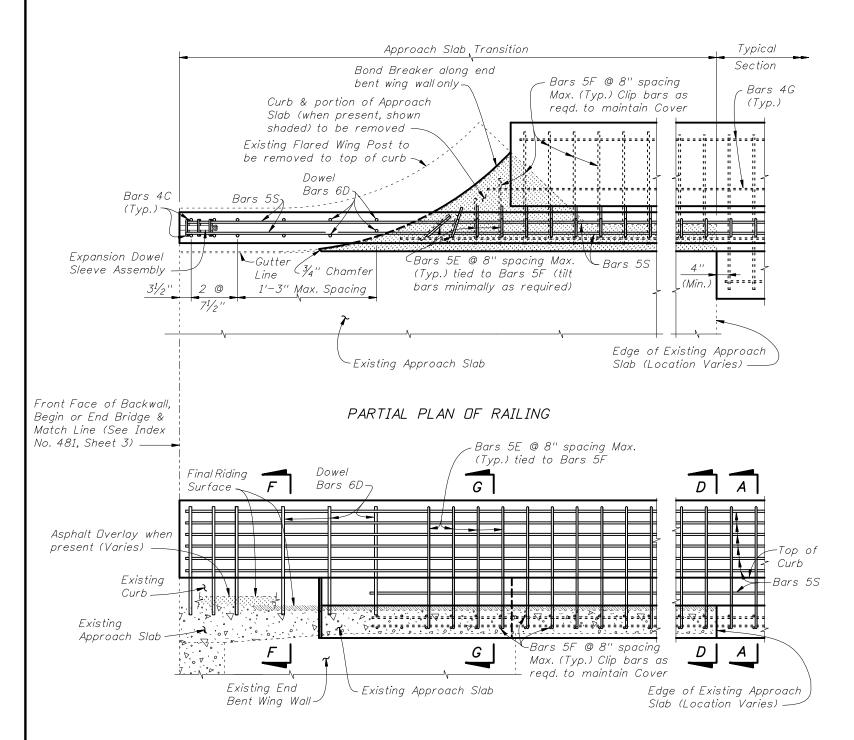
SECTION THRU EXISTING CURB AND APPROACH SLAB TO BE REMOVED (Free Standing Curb Similar)

CROSS REFERENCES:

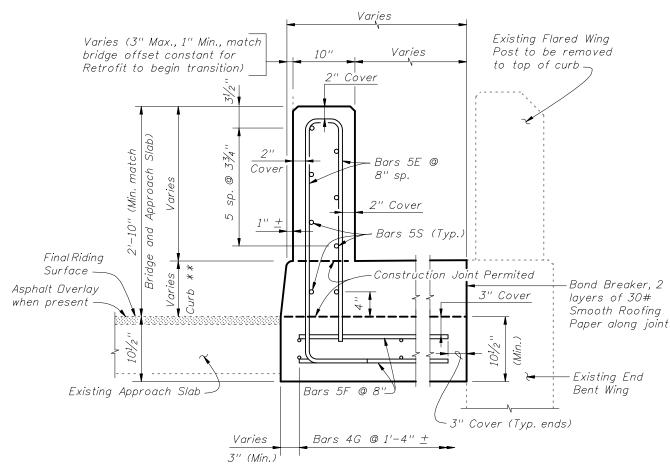
For Section A-A see Sheet 4.
For Section D-D see Sheet 5.
For Expansion Dowel Assembly and placement of Dowel Bars 6D Details see Index 480.



Sheet No.



== SCHEME 3 ~ MODIFICATION FOR INDEX NO.481 SCHEME 3 ====== RAILING END TREATMENT FOR FLARED WING WALLS WITH NARROW CURBS



SECTION G-G

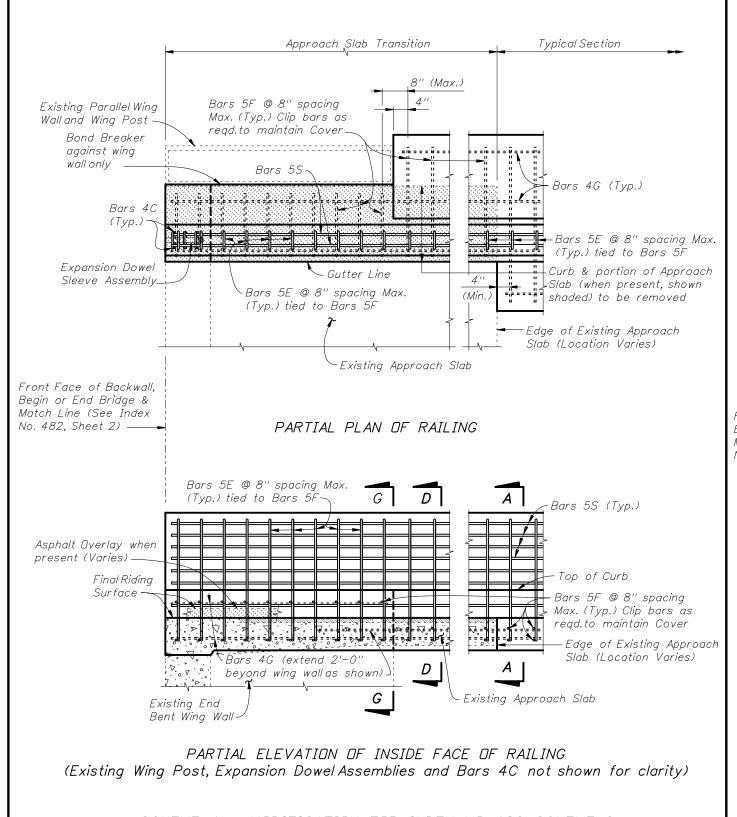
Note:

\*\* Match curb height at adjoining
existing end bent wing.

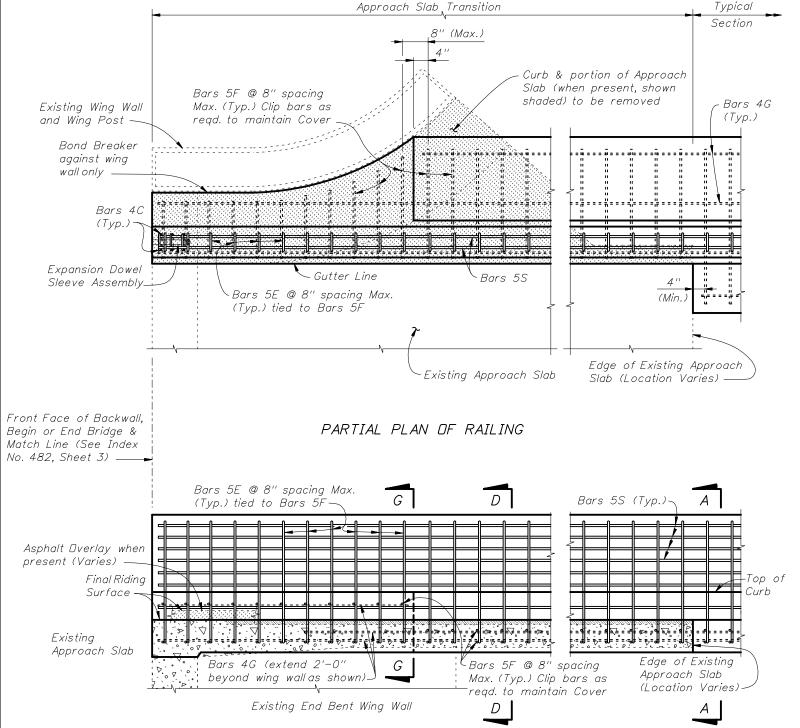
CROSS REFERENCES:

For Section A-A see Sheet 4.
For Section D-D see Sheet 5.
For Section F-F see Sheet 6.
For Expansion Dowel Assemblies Details and placement of Dowel Bars 6D see Index 480.





= SCHEME 4 ~ MODIFICATION FOR INDEX NO.482 SCHEME 2 ==== RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH WIDE CURBS



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Existing Wing Post, Expansion Dowel Assemblies and Bars 4C not shown for clarity)

=== SCHEME 5 ~ MODIFICATION FOR INDEX NO. 482 SCHEME 3 AND 4=== RAILING END TREATMENT FOR PARALLEL CURBS AND FLARED WING WALLS WITH WIDE CURBS

CROSS REFERENCES:

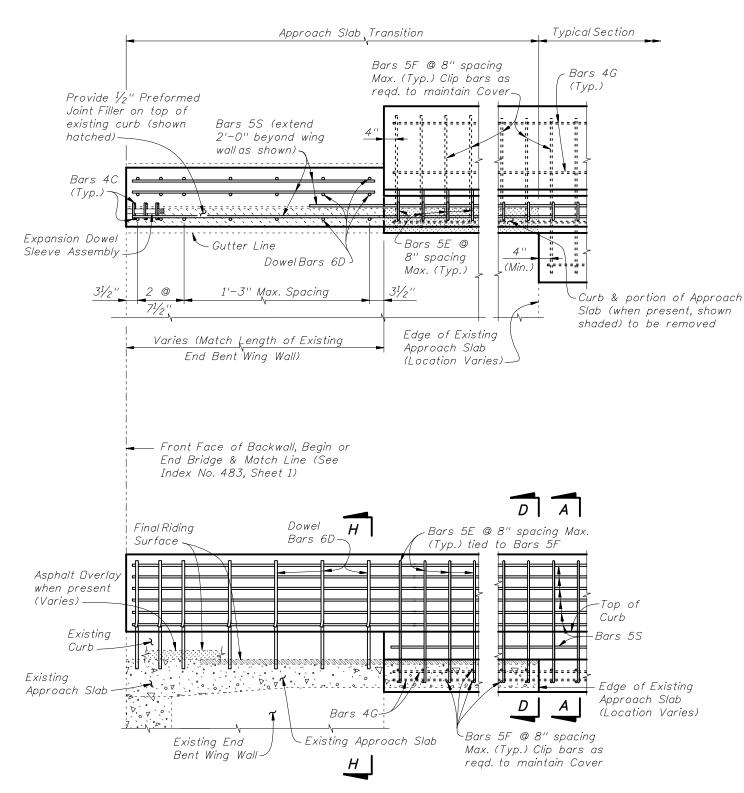
For Section A-A see Sheet 4 For Section D-D see Sheet 5. For Section G-G & H-H see Sheet 7. For Expansion Dowel Assemblies Details see Index 480.



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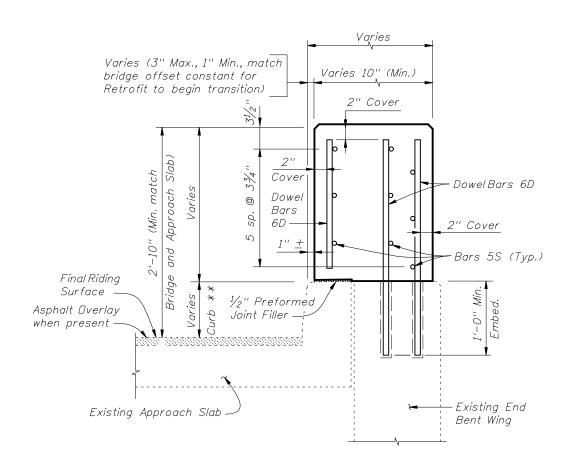
Sheet No.

1ndex No. 484



PARTIAL ELEVATION OF INSIDE FACE OF RAILING (Expansion Dowel Assemblies and Bars 4C not shown for clarity)

= SCHEME 6 ~ MODIFICATION FOR INDEX NO.483 SCHEME 2 ====== RAILING END TREATMENT FOR PARALLEL CURBS AND WING WALLS WITH INTERMEDIATE CURBS



SECTION H-H

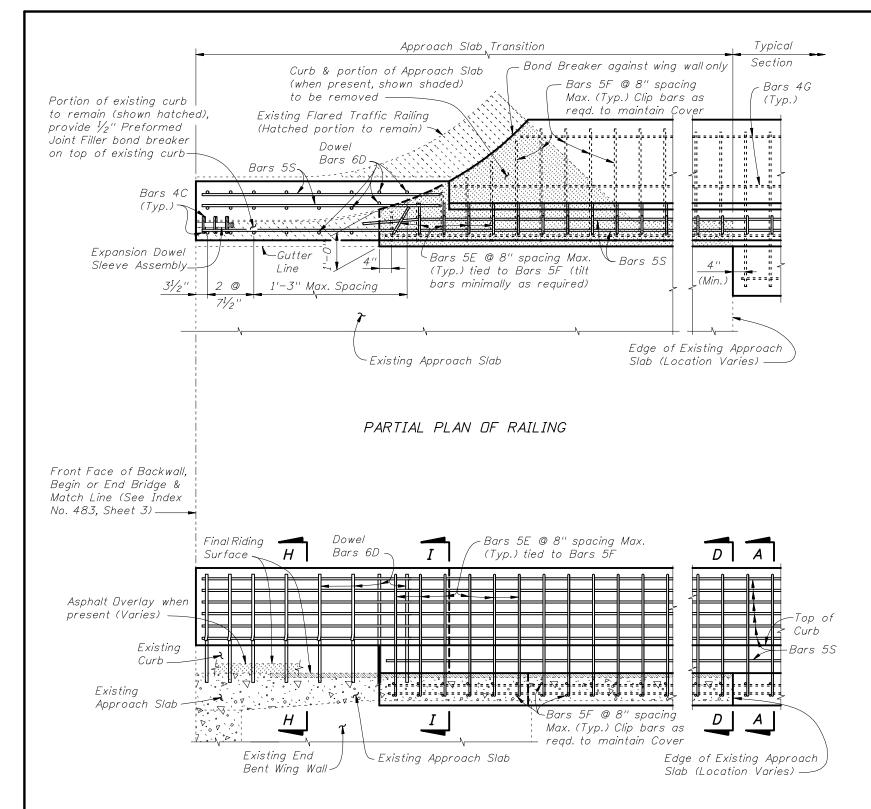
Note:

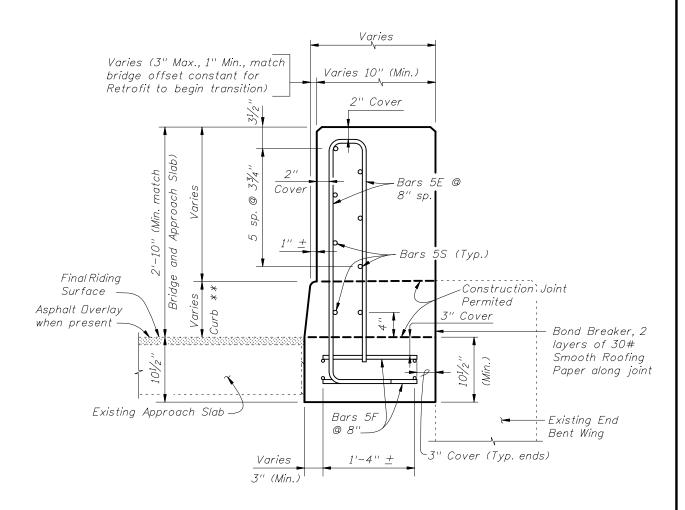
\*\* Match curb height at adjoining existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4.
For Section D-D see Sheet 5.
For Expansion Dowel Assembly
and placement of Dowel Bars 6D
Details see Index 480.







SECTION I-I

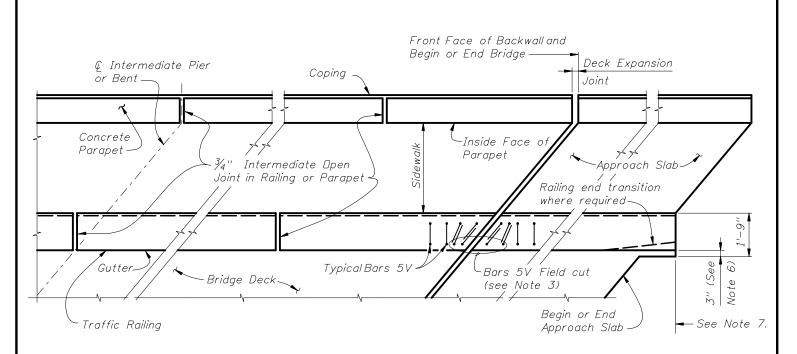
Note:

\*\* Match curb height at adjoining
existing end bent wing.

CROSS REFERENCES:

For Section A-A see Sheet 4.
For Section D-D see Sheet 5.
For Section H-H see Sheet 9.
For Expansion Dowel Assemblies and placement of Dowel Bars 6D Details see Index 480.

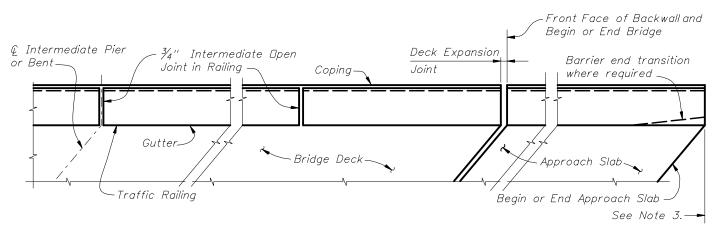




PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH SIDEWALK, TRAFFIC RAILING INDEX NO. 420 AND PEDESTRIAN/BICYCLE RAILING INDEX NO. 820, OTHER TRAFFIC RAILINGS SIMILAR

NOTES:

- 1) Concrete Parapet reinforcement is not effected by skew angle, see Index No. 820 for details.
- 2) Parapet expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 3) Traffic Railing reinforcement vertical Bars 5V & 5P may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement. Bars 5V adjacent to expansion joints shall be field adjusted to maintain clearance and spacing, extra Bars 5V will be required. Bars 5V bottom horizontal portion shall be cut so as to maintain maximum bottom horizontal length of bar to each vertical leg being placed, the remainder of bar shall be discarded. Cut Bars 5V may be rotated to maintain clearance.
- 4) Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. Expansion joint at the inside face of parapet shall be turned perpendicular or radial to this line. See Structures Plans, Superstructure and Approach Slab Sheets for details.
- 5) ¾" Intermediate Open Joints and V-Grooves in railing and parapet shall be placed perpendicular or radial to the gutter line or inside face of parapet line. See Structures Plans, Superstructure Sheets for locations.
- 6) At begin or end approach slab extend slab at the railing ends 3" (gutter side or back face of railing as required) as shown to provide a base for casting of the railing.
- 7) Begin placing Railing Bars 5P and 5V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.



PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH TRAFFIC RAILING INDEX NO. 420 SHOWN, OTHER TRAFFIC RAILINGS SIMILAR

NOTES:

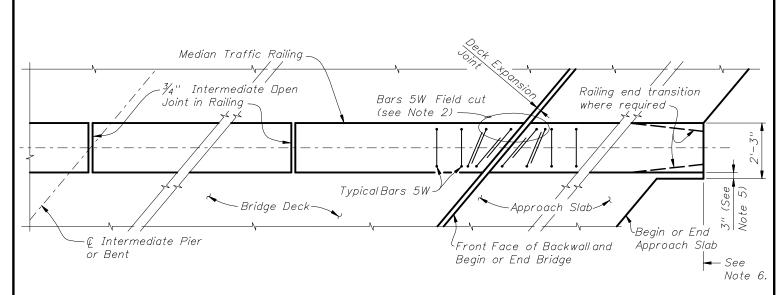
- 1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details.
- 2)  $\frac{3}{4}$ " Intermediate Open Joints and  $\frac{1}{2}$ " V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 3) Begin placing Railing Bars 5P and 5V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5P and 5V shall be made immediately adjacent to Begin or End Bridge.

GENERAL NOTES:

- 1) Work this Sheet with Traffic Railing, Pedestrian/Bicycle Railing, and Approach Slab Indexes as applicable.
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) Railings on Raised Sidewalks shall be treated similar to the Partial Plan View of Bridge Deck with Traffic Railing Index No. 420 Detail shown in the upper right corner of this sheet.
- 5) If Welded Wire Fabric is used in lieu of conventional reinforcement placement of the WWF vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible.



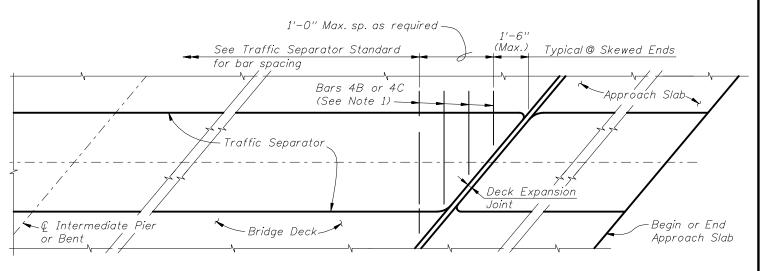
Sheet No.



### PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH MEDIAN TRAFFIC RAILING INDEX NO. 421

#### NOTES:

- 1) Median Traffic Railing reinforcement vertical Bars 5W may be shifted up to 1" (Max.) and rotated up to 10 degrees as required to allow proper placement.
- 2) Transition Stirrup Bars 5W shall be used as required at railing ends adjacent to expansion joints to facilitate placement of bars in acute corners. Place Transition Bars 5W in a fan pattern to maintain spacing. Rotate bars in 10° (Max.) increments as required.
- 3) Median Traffic Railing ends at deck expansion joints shall follow the deck joint with allowance for joint movement. See Structures Plans, Superstructure and Approach Slab Sheets for Details.
- 4)  $\frac{3}{4}$ " Intermediate Open Joints and  $\frac{1}{2}$ " V-Grooves in railing shall be placed perpendicular or radial to the  $\ell$  of the median railing. See Structures Plans, Superstructure and Approach Slab Sheets for locations.
- 5) At begin or end approach slab extend slab at the median railing ends 3" (open side) as shown to provide a base for casting of the railing.
- 6) Begin placing Railing Bars 5R and 5W on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 5R and 5W shall be made immediately adjacent to Begin or End Bridge.



### PARTIAL PLAN VIEW OF BRIDGE DECK AND APPROACH SLAB WITH TRAFFIC SEPARATOR INDEX NO. 302

#### NOTES:

- 1) Traffic Separator transverse reinforcement adjacent to deck expansion joints shall be field adjusted to maintain clearance and spacing. Bars shall be field cut as shown, bars may be rotated to maintain clearance.
- 2) Traffic Separator ends at deck expansion joints shall follow the deck joint limits. Drainage joints and  $\frac{1}{2}$ " V-Grooves shall be placed perpendicular or radial to the Q of the Traffic Separator. See Structures Plans, Superstructure and Approach Slab Sheets for details.

#### GENERAL NOTES:

- 1) Work this Sheet with Median Traffic Railing and Traffic Separator and Approach Slab Indexes as applicable.
- 2) Deck Expansion Joint at begin or end bridge shown. Deck Expansion Joints at © Pier or Intermediate Bents are similar.
- 3) Partial Plan Views shown are intended as guides only. See Structures Plans, Superstructure and Approach Slab Sheets for skew angles, joint orientation, dimensions and details.
- 4) If Welded Wire Fabric is used in lieu of conventional reinforcement placement of the WWF vertical elements shall be similar to those shown above. Clipping of horizontal elements to facilitate placement shall be minimized where possible.



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SKEW DETAILS FOR TRAFFIC RAILINGS, PARAPETS AND TRAFFIC SEPARATORS

1ndex No. 490