

ELEVATION

Wire Entry Conduit-

Drilled Shaft (See Details)

-To Roadway— 🗕

SELECTION PROCEDURE

- I. Determine the required pole height and bending moment at the pole base using a design wind speed in conformance with the "Plans Preparation Manual", Chapter 29, with a 30 percent gust factor.
- 2. Enter the Pole Moment Capacity Table, and determine the required Pole Type and wall thickness.
- 3. Enter the Pole Type and height designation in the signalization Plans for each strain pole. Example: From design: required height = 23'-6",

base moment = 198.0 kip-ft

From table use NS-VII-24

4. Refer to the Table of Variables for the required pole diameter, base plate and drilled shaft dimensions.

MI	VIMUM I	REQUIR	RED MO	MENT	CAPACI	TY (kip-	- <i>f†)</i>	
			TYF	E OF	POLE			
D (ft.)	NS-IV	NS-V	NS-VI	NS-VII	NS-VIII	NS-IX	NS-X	1
20	<i>33.</i> 0	106.0	152.0	210.0	266.0	330.0	390.0	1
22	<i>36.8</i>	III . 2	158,7	218.0	274.9	340.3	401.7	
24	40.6	116.4	165.3	226.0	283.9	350.7	4/3.3	
26	44.4	121.6	172.0	234.0	292.8	361.0	425.0	
28	<i>48</i> . 2	126.8	178.7	242.0	301.7	371.3	436.7	
30	52 . 0	132.0	185.3	250.0	3/0.7	38I . 7	448.3	
32	<i>55.8</i>	137.2	192.0	258.0	3/9.6	392.0	460.0	
34	<i>59.</i> 6	142.4	198.7	266.0	<i>328</i> , 5	402.3	471.7	
<i>3</i> 6	<i>63</i> . <i>4</i>	147.6	205.3	274.0	337.5	412.7	483.3	
38	<i>67</i> . 2	152.8	212.0	282.0	346.4	423.0	495.0	
40	71.0	<i>158.</i> 0	218.7	290.0	355.3	433.3	506.7	
42	74 . 8	163.2	225.3	298.0	364.3	443.7	5/8.3	
44	78 . 6	168.4	232.0	306.0	373.2	454.0	530.0	
4 6	82.4	<i>173.</i> 6	238.7	314.0	382./	464.3	541.7	
48	86.2	<i>178.8</i>	245 . 3	322.0	391.1	474.7	553.3	
50	90.0	184.0	252.0	330,0	400.0	485.0	565.0	

Aluminum Identification Tag Not to Exceed 2" x 4". Secure to Shaft by 0.125" Stainless Steel rivets or screws. Fabricators to provide details for approval. Identification Tag Located on inside of Pole visible from handhole. Tag to be stamped with the following information .

Financial Project ID Pole Type Pole Height Manufacturer's Name Certification No.

STEEL STRAIN POLE NOTES

I. Signal Structure Materials shall be as follows:

--> ASTM A607 Grade 50, 55 or 60 (less than $\frac{1}{4}$ ") or Poles ASTM A572 Grade 50 or 60 (1/4" and over) or ASTM A595 Grade A (55 ksi yleld) or Grade B (60 ksi yleld)

-> ASTM A709 Grade 36

Steel Plates Weld Metal *-> E70XX*

Bolts (except Anchor Bolts)-> ASTM A325, Type /

ASTM FI554 Grade 55 Anchor Bolts

Nuts for Anchor Bolts -> ASTM A563 Grade A Heavy Hex ASTM F436 Type I Washers for Anchor Bolts ->

-> ASTM A709 Grade 36 Handhole Frame

-> ASTM A607 Grade 50, 55 or 60 Handhole Cover

Aluminum Caps and Covers -> ASTM B26 (356-T6)

Stainless Steel Screws -> AISI Type 316

2. All Steel Items shall be Galvanized as follows:

All Nuts, Bolts and Washers ASTM AI53 Class C or D depending on size All other Steel Items -> ASTM AI23

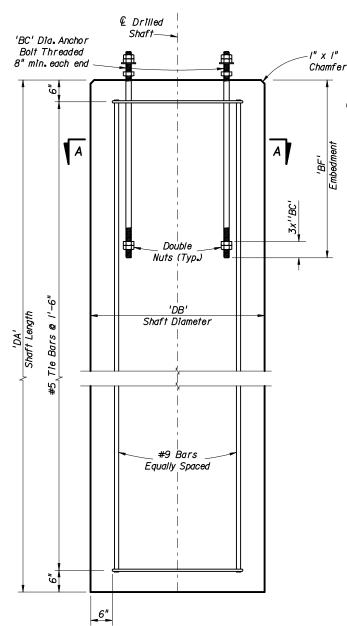
- 3. Concrete shall be Class IV (Drilled Shaft) with a minimum 28-day Compressive Strength (f'c) of 4,000 psi for all environmental classifications.
- 4. Reinforcina Steel shall be ASTM A615-96 Grade 60.
- 5. Grout shall have a mininum 28-day Compressive Strength of 5,000 psi and shall meet the requirements of Section 934. Grout after pole is set and properly plumbed.
- 6. A design wind speed of IOO mph with a 30% gust factor for wind loading on the pole was included in the design.
- 7. The Pole shall be tapered with the diameter changing at a rate of OJ4 inch per foot.
- 8. Except for anchor bolts, all bolt hole diameters shall be equal to the bolt diameter plus $\frac{1}{16}$, prior to galvanizing. Hole diameters for anchor bolts shall not exceed the bolt diameter plus 1/2".
- 9. The foundation for the Strain Pole Structure shall be constructed in accordance with Section 455 of the FDOT Specifications except that no payment for the foundation shall be made under Section 455. The cost of providing the foundation shall be included in the pay item for providing the complete Strain Pole Structure. For foundation design assumptions, refer to the Foundation Notes
- 10. The pole shall be free of transverse welds except at the base.
- II. Poles constructed out of two or more sections with overlapping splices are not permitted.
- 12. The strain pole shall not be erected until the foundation concrete has been allowed to cure for a minimum of seven days.
- 13. No field welding on any part of the pole is permitted.
- 14. For clamp spacing, cable sizes and forces, signal and sign mounting locations and details see the Signalization Plans.
- 15. All welding shall conform to American Welding Society Structural Welding Code (Steel) ANSI/AWS DIJ (current edition).
- 16. See Standard Index No.17727 for grounding detail and span wire installation details.
- 17. Locate handhole 180° from 2 inch wire entrance pipe.
- 18. Paint Steel Strain Poles in accordance with Section 649. Mast Arm Assemblies.

ELEVATION AND NOTES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STEEL STRAIN POLE

	Names	Dates	Approve	d By /) . つ	12/
Designed By			Stat	te Structures De	sign Engineer
Orawn By			Revision	Sheet No.	Index No.
Checked By			02	lof 3	<i>17723</i>



FOUNDATION NOTES:

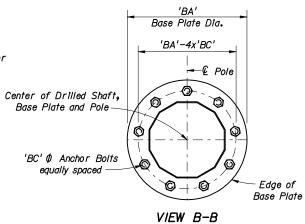
The foundations for Steel Strain Poles are pre-designed and are based upon the following conservative soil criteria which covers the great majority of soil types found in Florida.

DRILLED SHAFT ELEVATION (See Table for number of #9 bars.)

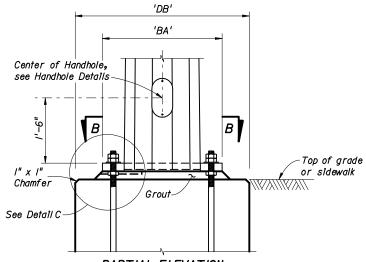
Classification = Cohesionless (Fine Sand)
Friction Angle = 30 Degrees (30°)

Unit Weight = 50 lbs./cu.ft.(assumed saturated)

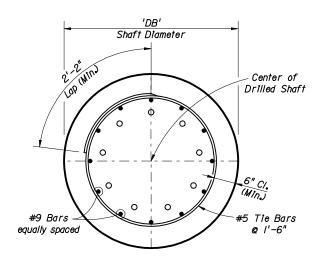
Only in cases where the Designer considers the soil types at the specific site location to be of lesser strength properties should an analysis be required. Auger borings, SPT borings or CPT soundings may be utilized as needed to verify the assumed soil properties, and at relatively uniform sites, a single boring or sounding may cover several foundations. Furthermore, borings in the area that were performed for the other purposes may be used to confirm the assumed soil properties.



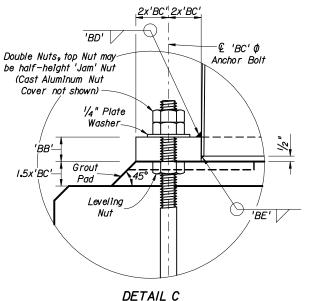
NOTE: Number of bolts shown for illustration purposes only. (See Tabel for actual quantity)

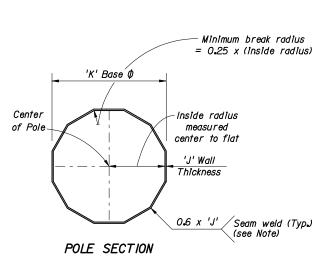


PARTIAL ELEVATION
(Showing Base Plate, Anchor bolts and Handhole)



SECTION A-A





NOTE: Longitudinal seam welds
within 6" of circumferential
welds shall be complete
penetration welds.

TABLE OF STRAIN POLE VARIABLES												
P0LE	POLE		BASE CONNECTION						SHAFT			
TYPE	J (în•)	K (în .)	No. of Bolts	BA (în.)	BB (în•)	BC (în.)	BD (în•)	BE (in.)	BF (în•)	DA (ft)	DB (ft)	No.of #9 bars
NS-IV		14	6	25	2,125	I .3 75	0.3/3	0.188	36	10	3. 5	14
NS-V		16	8	27	2,250	I . 375	0.375	0.188	47	12.5	3. 5	14
NS-VI	0.239	18	8	30	2,375	I . 500	0.438	0.188	5 4	14	3. 5	14
NS-VII	0 =	21	10	33	2,250	1 . 500	0.375	0.188	49	<i>1</i> 5	4	19
NS-VIII	ا ر	23	12	34	2,250	I . 375	0.375	0.188	52	16	4	19
NS-IX		25	12	37	2,250	1 . 500	0.375	0.188	50	16	4. 5	23
NS-X		27	12	39	2,375	1 . 500	0.375	0.188	52	17	4. 5	23
NS-V		<i>1</i> 6	8	28	2,375	1 . 500	0.438	0.250	47	12.5	3. 5	14
NS-VI	0.3/3	18	10	30	2,375	1.500	0.500	0.250	5 4	14	3. 5	14
NS-VII	J = 0	21	12	33	2.375	1.500	0.500	0.250	49	15	4	19
NS-VIII		23	12	3 5	2,500	1.500	0.500	0.250	52	16	4	19
NS-IX		25	12	39	2,625	1 . 750	0.500	0.250	50	16	4. 5	23
NS-X		27	12	41	2,750	1.750	0,500	0.250	52	17	4. 5	23

Note: Details shown on this sheet are for I2 sided pole sections. However, sections with more than I2 sides and round sections are permitted, provided the outside diameter and well thickness are not reduced.

BASE DETAILS AND TABLE OF VARIABLES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STEEL STRAIN POLE

	Names	Dates	Approve	d By //)	Ω	12/
Designed By			Stat	e Structure	s De	sign Engineer
Drawn By			Revision	Sheet	No.	Index No.
Checked By			02	2 of	3	17723

