

ROADWAY AND TRAFFIC DESIGN STANDARDS

FOR DESIGN, CONSTRUCTION, MAINTENANCE AND UTILITY OPERATIONS ON THE STATE HIGHWAY SYSTEM

JANUARY 2000

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Approved For Use On Federal Aid Projects

For James E. St. John, Division Administrator

ENGLISH UNITS

Billy L. Hattaway State Roadway Design Engineer State Of Florida, Department Of Transportation Roadway Design Office, Mail Station 32

605 Suwannee Street

Tallahassee, Florida 32399-0450

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INTRODUCTION

ROADWAY AND TRAFFIC DESIGN STANDARDS, JANUARY 2000

The 2000 English Roadway And Traffic Design Standards booklet was developed in concert with the development of the Department's roadway "Plans Preparation Manual", current as of January 2000; the 2000 "FDOT Standard Specifications For Road And Bridge Construction"; the current edition of the FDOT "Basis Of Estimate Handbook, English And Metric Combined Edition"; the AASHTO metric "A Policy On Geometric Design Of Highways And Streets", 1994; the AASHTO "Roadside Design Guide", 1996; the USDOT (FHWA) "Manual On Uniform Traffic Control Devices", 1988 and revisions; the USDOT rules implementing the Americans With Disabilities Act (ADA) and its proposed amendments to the rules, June 20, 1994; the "Florida Accessibility Code For Building Construction" (10/97 Edition); ASTM and AASHTO specifications; and, obtainable industry established interim and proposed specifications and trade indentifications.

Users of these standards are encouraged to suggest improvements to the standards. Suggestions should be submitted only after thorough study has been made and supporting background data can be furnished with the suggestion. Suggestions should be submitted to the State Roadway Design Engineer, Florida Department Of Transportation, 605 Suwannee St. MS 32, Tallahassee, FL. 32399-0450. The Department will make changes to the standards by special provisions, or where necessary by interim standards, until the next publication of standards is implemented.

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Area or Amperes Design Hour Factor or Kelvin 444 American Automobile Association Cl. or Clear CL, C/L or € Clearance Center Line F or Final Final Quantity Kilo (prefix) AASHO American Association Of State Highway Officials Furnish & Install Kilogram Kilogram Per Meter Face to Face Federal Aid or Fine Aggregate American Association Of State Highway And Transportation Officials AASHT0 Concrete Monument F to F CMB kg/m² kg/m³ Kilo ABC Concrete Median Barrier FA Kilogram Per Square Meter Asphalt Base Course Abandoned
Acrylonitrite-Butadiene-Styrene Pipe CMP CMPA Corrugated Metal Pipe FAC FAP Florida Administrative Code Federal Aid Project Kilogram Per Cubic Meter Corrugated Metal Pipe Arch ARS One Thousand Kip km km/h AC, Ac. Acre
AC or Asph. Conc. Asphaltic Concrete Co. Col. FC FD Friction Course 1000 Pounds Column French Drain Kilometer Accel. Acceleration Commercial or Common Fdn. Foundation Kilometer Per Hour Florida Department Of Transportation Act. ADA Actuated COMM Committee or By Committee FDOT FE kn kPa The Americans With Disabilities Act Composite Comp. Kilopascal Floor Elevation Connect or Connection Adh. Adhesive Con. Conc. Fed. Fert. Federal Kips Per Square Inch Adj. ADT Adiust Concrete Fertilizer kV Kilovolt Average Daily Traffic Construct or Construction Kilovolt Ampere FES FETS Flared End Section Annual Average Daily Traffic AADT Contrl. Controller Flared End Terminal Section kWh Kilowatthour Continuation Aggregate Agg. Ah. AISC FΗ Fire Hydrant Federal Highway Administration Contr Contractor FHWA Length, Length Of Curve, Liter, Left American Institute Of Steel Construction Coord. Coordinate L 2-L Figure Finish Two-Lane Two-Lane One-Way Alternate Cor. Corr. Corner 2LIW Corrugated Aluminum Flow Line 2L2W LA or L/A Two-Lane Two-Way Limited Access 12:00 Midnight Until II:59 Noon CP CPE FL. FI. or Fla. Florida American National Standards Institute Corrugated Polyethylene Pine ANSI AOS Flexible Flex. Lane Kilometer Apparent Opening Size CR CRA Control Radius or County Road Fuse (Type Slow Burn) FNO Lat. Lb. LBR LC Lateral or Latitude FOC FPM or form Applied, Application Clear Recovery Area Fiber Optics Cable Crs. or Cse. Course Apprh. Approach Feet Per Minute Curve To Spiral Limerock Bearing Ratio CS CSP CT Approx ARTBA *Approximate* FPS or fps Feet Per Second Long Chord American Road & Transportation Builders Association Corrugated Steel Pipe FR or Fr. Lgth. Lin. Length Artf. Asph. Artificial Clear Trunk Frang. Franaible Linear **Asphalt** Ctlvr. Cantilever Freq. Frequency lm Lmrk. Assem. Assembly Center FS Far Side Limerock Association Assn. CII or Cu Conner Ft. FTB Foot or Feet Loc., LO Location Assoc. Associate, Association Culvert Floating Turbidity Barrier American Society For Testing Materials Length Of Spiral Left Turn ASTM Cwt. CY Hundredweiaht FTBA Florida Transportation Builder Association Attn. Cubic Yard Furn. Fut. Lt. Ltd. Attnuatr. Attenuator Cylindrical Future Lighted or Limited Auxiliary Aux. or Auxil. Clear Zone Lum. L/W Ix Avenue G Giga or Gauss Lightweight American Wire Gauge AWG Ω Degree Of Curvature, Depth, Density, Distance, Diameter Gram or Gravity AWS American Welding Society or Directional Distribution Az. Azimuth DA Drainage Area or Deflection Angle Ga. Ga. or Gal. Gauge or Gage Mass, Middle Ordinate Length or Mega DBH Diameter At Breast Height B to B Back to Back m m² m³ m³/m Meter or Milli DBI DbI. Ditch Bottom Inlet Basc. Bascule Gar GD GarageSquare Meter or Meter Square Cubic Meter or Meter Cubed Double | Barrel Gutter Drain DCS Degree Of Curvature (Spiral) Bd. or Bnd. Bond or Bonded GIP GM GP Galvanized Iron Pipe Cubic Meter Per Meter DD DDHV Bottle Cap or Bolt Circle Dry Density Directional Design Hour Traffic Gas Main m/s Meters Per Second B/C. B.C. Back Of Curb
Bituminous Coated Corrugated Metal Pipe Culvert Grade Point Decel. Deceleration Grade, Guardrail or Grate BCCMP Maint Maintenance Bituminous Coated Pipe Arch Culvert Bituminous Coated And Paved Corrugated Metal Pipe Culvert **BCPA** Dearee Gr. or Gro. Gross Mat/. Material Delineators **BCPCMF** Galvanized Rigid Steel Conduit GRC Grd. Max. MB MBM Med. Maximum BCPPA BCT BE Demobilization Bituminous Coated And Paved Pipe Arch Culvert Demoh! Ground Median Barrier Department Breakaway Cable Terminal Buried Electric gross km Gr. Wt. or gr. wt. Gross Kilometer Thousand (Feet) Board Measure Detour, Detection, Detectable Gross Weight DGN or Dgn. Median Design Beg. Bit. Begin Bituminous Mega One Million Design Hourly Volume DHV Gy Gray Memb. MES Member Mitered End Section DHW DT Design High Water Bk. BL, BLC Bldg. Base Line, Base Line Control Mess. Mfg. MG MH Message Drop Inlet Building Manufactured or Manufacturer Dia. or D Diameter Hour or Hecto RIkhd Bulkhead 1000 Gallons Hectare Blvd. BM Dim. Dist. Dimension Boulevard HB Hay Bales Manhale Distance Bench Mark Mean High Water HC Horizontal Clearance Disp DLS Disposal Bndry. Boundary District Location Surveyor High Density or Heavy Duty Micro Bdr. Bot. DMM DOT Domestic Mail Manual
Department Of Transportation HD or Hd. Head Rottom One - Millionth Headwall Micro Hdw/. Borrow Pit Ditch Point Intersection Mid. НН Heavy Hex Middle Bq. Br. Becquere/ Drain, Drive or Design Review Mil Mil. One-Thousandth Of An Inch Dr. or DR. DR Hndrl Handrail Bridae Design Review HOA Hand/Off/Automatic Military Brg. Brkwy. Milli One - Thousandth Horiz, or Hor. Driv. Horizontal Breakaway High Pressure or Horsepower Min. Misc. Minimum or Minute Buried Telephone Cable or Duct Drwy Driveway Miscellaneous Hr. Btfly. Design Speed Rutterfly High Strength mL MLW Milliliter Design Service Life Drawing Barbed Wire, Bottom Width or Both Ways DSI Hse. Mean Low Water Dwg. B0 Heiaht High Water or Hot Water Mobil HW Mobilization East or External Distance Cantilever Length, Cut. Colorless, Coulomb or Cycle Length Mod. Modify or Modified Highway Hydrant or Hydraulic Hwy. Rate Of Superelevation Degree Celsius Curb And Gutter Mol Mon. MOT MP Hyd. Hz Mole E to E End to End C & G Monument EA or Ea. Coarse Aggregate Maintenance Of Traffic Fastbound Capacity Corrugated Aluminum Pipe Cap. CAP Mile Post External Angle (Delta), Interstate El. or Elev. Elevation MPa Megapasca/ Interchange
Illuminating Engineering Society Elastomeric Intchg. or Ichg. Caps. CASP CATV Capital Letters Corrugated Aluminized Steel Pipe MPH or mob Miles Per Hour IES Elec. Electric MSL Mean Sea Level Ellip. Embk. Elliptical Inside Diameter or Identification Intermediate Metal Conduit Cable Television Mtd. Mounted IMC Embankment CB Manual On Uniform Traffic Control Device Catch Basin MUTCD Emulsified Emul. CBC CBS Concrete Box Culvert Manual On Uniform Traffic Studies Incorporated or Including Encl. Engr. EOS Enclosure Inc. Concrete Box Structure Incl. or Inc. Included Enaineer Center to Center STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION End Of Survey or Equivalent Opening Size Industry or Industrial Ind. IP Center to Center Fach Way CCFW Iron Pipe CD Ea. Equation or Equal Cross Drain Equipment Install. Installed cd Cem. Candela Intersection Cement or Cemetery F.smt. Fasement Isect. IsI. ITE Cem'd. CFS Establish or Established Institute Of Transportation Engineers STANDARD ABBREVIATIONS Cubic Feet Per Second Etc. or etc. Et Cetera (And So Forth) Channel Channel Change Chchg. Ex. Except Chg. Junction Box Changeable JB Excavation Approved By Names Dates Bill J. Author Cast Iron Jct. Existing Expansion Exist. Cast Iron Pipe Jt. Joint Designed By Cast In Place EXD. CIPL State Roadway Design Enginee

Extension

Expressway

Exwy.

cir. or circ.

Circle or Circular

Circumference

1 of 2

00

rawn By

hecked By

Tangent, Length Of Curve, Percent Trucks, Tesla, Radius T, TWP or Twp. Township
t Metric Ton N/m Newtons Per Meter Newtons Per Square Meter R or Rng. Range Radian N/m^2 rad Newtons Per Cubic Meter Newtons Per Square Millimeter Radian Per Second Rock Base Asphaltic Concrete N/m3 tan. TBM Temporary Bench Mark Tangent To Curve N/mm² RBAC TC TCB TCE TCP TCZ Not Available or Not Applicable RBST Rock Base Surface Treatment Temporary Concrete Barrier N & C Nail & Cap RCP Reinforced Concrete Pipe RCPA Reinforced Concrete Pipe Arch Temporary Construction Easement Northbound Terra Cotta Pipe Traffic Control Zone National Coarse Rd. Road or Round Neighborhood Delivery And Collection Bax Unit NDCBU Rdsd. Roadside Roadway Telephone Temperature or Temporary net km Net Kilometer Rec. Recovery Temp. NEMA National Electrical Manufacturers Association Reticuline or Rectangular NGVD National Geodetic Vertical Datum of 1929 Ref. Reference Theo. Theoretical NGS National Geodetic Survey Ref1. THRMPLSTC Thermoplastic Reflective Insulation (Flame Retardant, Moisture And Heat Resistant Thermoplastic)
Thickness NHW Normal High Water Reg. Reinf. Region, Regular, Registered or Regulation Reinforced or Reinforcing THW or THWN Thick. NIC Not In Contract NJ Rejuv. Rejuvenation Thick, Thickness or Truck New Jersey Nam Newton Meter Reloc. Relocated Trans. Transition, Transverse, Translate or Transportation No. Number Removal Replace Required Nom. Nominal Repl. Treat. Treatment Req. or Reqd. Tangent To Spiral Norm. NS NT.N&T TSC Length Of Tangent (Spiral Curve) Non Stress, Not Suitable or Near Side Res. Residence or Residential Insulation (Moisture & Heat Resistant Rubber) Non Traffic, Nail & Tin RHW Typ.Typical Not To Scale Reference Monument r/min NWNorthwest Revolution Per Minute Upass. ${\it Underpass}$ Reference Point Underground Revolution Per Minute rрт RРМ Overpass Out to Out Underwriters Laboratories UL Uit. Raised Reflective Pavement Markers O to O or o to o *Ultimate* r/s RR Revolution Per Second UItd. Overall Railroad 0.B.G. Optional Base Group Unddr. ${\it Underdrains}$ Rsf. Resurface On Center Underroadway Undrdwy. OD Outside Diameter UNL or Undl. Unloaded R/W, ROW Right Of Way Overhead Electric Untreated OH, OHD or Ohd. USC & GS US Coast and Geodetic Survey (now National Geodetic Survey) Option, Optional or Optically Sors Speed, South, Seimens, Or Second US Geological Survey United States Postal Service Opt. OT USGS Overhead Telephone SAHM Sand - Asphalt Hot Mix USPS SAN or San. Sanitary Southbound Oz. Ounce Utilities SB SBAC SBRM Shell Base Asphaltic Concrete Volt. Velocity, Volume or Hourly Volume Sand Bituminous Road Mix Varies, Variable or Variance Vertical Curve SBRM SBST SC Sch. SCST SD SE Sec. Var. Passenger Car & Light Delivery Truck Shell Base Surface Treatment P or Plan Plan Quantity Seal Coat or Spiral To Curve VCP VECP Vitrified Clay Pipe Schedule Pa Pascal Value Engineering Change Proposal Sand-Clay Surface Treatment Side Drain, Storm Drain Parallel Pa•s Pascal Second Vert. Vertical Part. Participation or Partition Vertical Foot Pavt. PC Second Vh VMS Verified Horizontal Location Sect. Point Of Curvature Variable Message Sign PCBC Precast Concrete Box Culvert Sed. Sep. Sediment Volume Vertical Panel Point Of Compound Curvature or Plain Cement Concrete Separator PCC PCE Permanent Construction Easement Seq. Serv. Sequential VPD or Vpd. Vehicles Per Day PE Ped Professional Engineer Service VPH or Vph. VPHPL or Vphpl. Vehicles Per Hour Pedestrian or Pedestal Adjustment Factor In Percent, Silt Fence Vehicles Per Hour Per Lane SG SG or Sp.Gr. Subgrade Specific Gravity Pen. Penetration Volts Root Mean Square Verified Vertical Elevation VRMS Profile Grade PG PGL Profile Grade Line Sh. or Sht. Sheet Verified Vertical Elevation And Horizontal Location Shidr. Shoulder Ph. Phase VWVariable Width Measure Of Acidity or Alkalinity SHW Seasonal High Water Point Of Intersection Spa. Space Spcg. or Sp. Width, Wide, West or Watt Pkg. Parking Specification W/C Water - Cement Ratio Sq. Ft. or SF Square Foot Westbound PL or P Property Line or Plate Sq. In.
Sq. Yd. or SY
SR or S.R.
SRAP PM POC 12:00 Noon Until II:59 Midnight Square Inch Square Yard Wb. WB40 Weher Intermediate Semi Trailer Point On Curve POST POT State Road Spiral Rib Aluminum Pipe Large Semi Trailer Tandem Semi Trailer Point On Semi-Tangent WB50 Point On Tangent WB60 SRASP Spiral Rib Aluminized Steel Pipe Water Main Power Pole Pr. PRC Spiral Rib Steel Pine W.P./. Work Program Item SRSP SS SSMD Point Of Reverse Curvature Sanitary Sewer Solid State Modular Design Surface Treatment or Spiral To Tangent Prcst. WWF Welded Wire Fabric Prestressed Prest. Probability
Product, Production, Producer or Produced
Program or Progression St. or ST. Street Prob. Coordinate Value (East-West Direction) or Extra Prod. Sta. Station X Rd. Cross Road Stab. STB Std. Prog. Proj. PRM Stability or Stabilization Crossing Xing. Project or Projection
Permanent Reference Monument Staked Turbidity Barrier Xsec. Cross Section Standard Stg. Stge. Stl. Str. Provisions Strong Plans, Specifications And Estimates Coordinate Value (North-South Direction) PS & E Storage PSF or psf Pounds Per Square Foot PSI or psi Pounds Per Square Inch Structure Point Of Tangency Sty. SU Single Unit Trucks PVC Polyvinyl Chloride Sub. or Subs. Pressure Water Sub. or Subst. Substitute

Subar.

SW

Suppts. SUR or Sur.

Sys. or Syst.

Subarade

Supports

Survey Surface

System Sievert

Southwest Sidewalk

 \bigcirc

Ω

Peak Discharge or Flow Volume

UNITS OF MEASURE

US	MEASU	IREME	NT.	
Α	C Acr	e		
Α	S Ass	embly		
В	U Bus	hel ´		
	F Cub		†	
	O Cle			
С	Y Cub	ic Yar	d	
Ε	A Eac	h		
Ε	D Eac	h Day		
G	A Gall	on		
G	M Gros	ss Mil	е	
	B Pou			
L	F Line	ear Fo	ot	
L	M Lan	e Mile	•	
L	0 Per	Locat	ion	
L	S Lum	p Sum	,	
L	U Lum	inaire		
М	B Tho	usand	Board	Meası
	'G Tho		Gallons	ì
	H Man			
N	M Net	Mile		
	'A Per			
	B Per		ing	
	E Pile			
	l Per		section	
	L Plai			
	M Per			
	'S Per			
	W Per			
Ş	F Squ	are F	oot	
	Y Squ		ard	
T	N Ton			

METRIC MEASUREMENT

METRIC MEASUREMENT	
METRIC MEASUREMENT AS Assembly CO Cleanout DA Day EA Each ED Each Day GK Gross Kilometer HA Hectare HR HOUT KG Kilogram KL Kiloliter KM Kilometer LI Liter LK Lane Kilometer LO PER LOCATION LS Lump Sum PER ASSEMBLY LS/AS Lump Sum PER Day LS/EA Lump Sum PER Day LS/EA Lump Sum PER CACH LS/HA Lump Sum PER Hectare LS/HA Lump Sum PER Kilogram LS/LS Lump Sum PER Ling Sum LS/LS Lump Sum PER Ling Sum LS/LS Lump Sum PER Lump Sum LS/MI Lump Sum PER Metric Ton LS/MI Lump Sum PER Square Met LU Luminaire MH Man Hour MO Month MT Metric Ton MI Meter Square Meter M2 Square Meter M3 Subic Meter	er

Plant

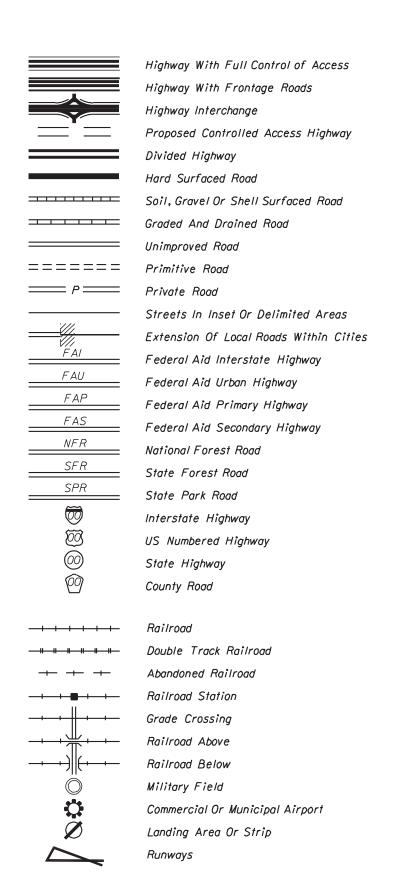
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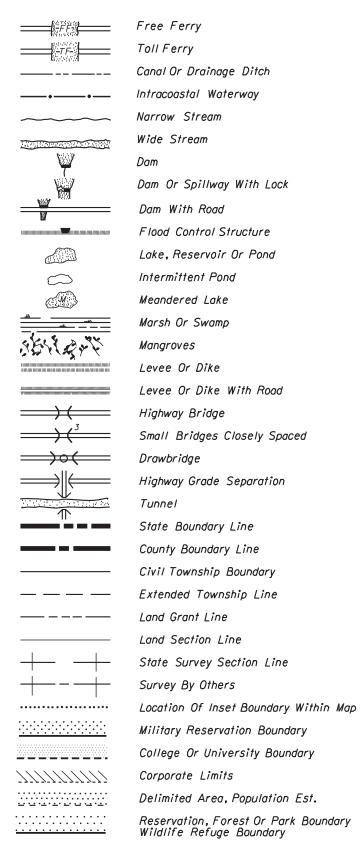
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

STANDARD ABBREVIATIONS

	Names	Dates	Approve	d By J. n	P-71
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	2 of 2	007

STANDARD SYMBOLS FOR KEY MAP





	Residential Area Under Development
*	Lighthouse
	State Capital
	County Seat
0	Other City Or Village
X	Seminole Indian Village
☆	Welcome Station
W <i>P</i>	Wayside Park Or Small Park
-w <i>F</i> -	Park With Boat Ramp
- β -	Boat Ramp
	Museum
A	Recreational Area Or Historic Site
	Scenic Site
	Post Office
Ė	School
	Church
1 <u>+</u> 1	Cemetery
	Church And Cemetery
+	Hospital, Health Center Or Rest Home
	Toll House, Port Of Entry Or Weight Station
	Fair Grounds, Race Course Or Rodeo Arena
*	Mine Or Strip Mine
•	Governmental Research Station

	Agricultural Inspection Station
FM	Farmers Market
ldot	Game Preserve
+	Game Checking Station
4	Bird Sanctuary
4	Fire Control Headquarters
	Lookout Tower
FS	Fire Station
*	Patrol Or Police Station
	Correctional Institution Or Road Camp
DOT	Department of Transportation Facility
+	Coast Guard Station
	Armory
J	Junkyard
F	Sanitary Fill
s	Sewage Disposal Plant
1	Incinerator
4	Power Plant
\bigcap	Power Substation
<u>د</u>	Communications Facility
-××-	Locked Gate Or Fence
wood <u></u> ▲	Triangulation Station

GENERAL NOTE

I. Symbols on this Index are intended for use on all Roadway, Signing And Marking, Signalization, and Lighting projects. For work zone traffic control symbols refer to Index 600. When additional or similar symbols are used, legends or notations may be required for clarity.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STANDARD SYMBOLS

	Names	Dates	Approve	d By R.m.	→ ¬ ,,
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	1 of 3	002

STANDARD SYMBOLS FOR PLAN SHEETS LITHITY ADJUSTMENT SYMBOLS

	GENER	AL SYMB	OLS		UTILITY	ADJUSTMENT S	YMBOLS		
•	State Line		Curb	EXISTING	PROPOSED		EXISTING	PROPOSED	
	•		Curb And Gutter	(E).		Manhala	^	^	
	Township Line	@ →\ >	Water Well, Spring		© -	Manhole		→	Power Pole
	Section Line		Levee	(.) 	đ	Fire Hydrant			Telephone Pole
	City Line	Ŷ ^{MP} 327	Railroad Mile Post	LJ		Meter (Type)		- ♦	Combination Pole
	Base Or Survey Line		Railroad Signal With Gate	{><}	->>-	Valve (Type)	}	\rightarrow	Guy Wire And Anchor Pin
	Right -Of - Way	Î	Railroad Switch	[>	->-	Valve Box (Type)	0		Guy Pole Deadman
•	Easement Line	— X 12' X —	Gate		-⊗-	Valve Cover (Type)	\$57.49 \$60.58		Tower
•	Limited Access Line	0 0	Pump Island	*/#=*/>	⇔	Vent (Type)	O0	\bigcirc	Light Pole
•	Fence Line		Storage Tank (Surface)	M -		Pump Station	*CONTRACT *CONTRACT		Transformer
	National Or State Park Or Forest	(22)	Storage Tank (Underground)	<u> </u>	\$	Sewage Pump Station	^		
	Grant Line		Mine Or Quarry	0		Cleanout			Overhead Electric
	Railroad (Drainage Maps)	ВР	Borrow Pit			Cable TV Service Box	de (7.5 kV) de	DE DE (1.5 KV) DE DE	
	Railroad (Detail Plans)	†	Church		H	CODIC IV SCIVICE BOX	O OT (2")O	—O— OT (2") —O—	
	Fence (Limited Access)	<u> </u>	Store	6" Gas	6" Gas	Gas	от 2" от	ататат 2" ататат	Overhead Telephone
	Box Culvert	RES	Residence	G G - 6" G -	GGG 6" GGGGGGG				
	Bridge	В	Barn	6" Water	———— 6" Water ———	141 - 4 44 - 4 -	— - OH (2" CATV) —	—— OH (2" CATV) ——	Overhead Cable Television
	Pipe Culvert-Mitered End Section	•	School	w w - 6" w -	www6"wwwwwwww	Water Main	0102 010	01 0 01 0 2 01 0 01 0	
	Pipe Culvert-Straight Endwall	• • • •	Hay Bales		a.u. a			——— OFO (I")———	Overhead Fiber Optic
I ————————————————————————————————————	Pipe Culvert-U-Type Endwall		Silt Fence		8" San sss 8"sssssss	Sanitary Sewer	0F0 /" 0F0	ofo ofo /" ofo of	Sovernedd Fiber Opiic
I	Pipe Culvert-Median Drain		Floating Turbidity Barrier	55-0 5-	2220222222	-			
	Pipe Culvert-Other End Treatments		Staked Turbidity Barrier	BE (7.5 kV)	——BE (7.5 kV)——	Buried Electric			
18" SD	Storm Drain			BE - (7.5 kV) BE -	BEBE(7.5 kV)BEBE	Borred Erectric			
18" SD	Storm Drain		Shore Line	07 (01)	0.7 / 0// 1				
					——— <i>ВТ (2")</i> ——— втвтвт <i>2</i> " втвт	Buried Telephone			
	Manhole	عللد عللد عللد عللد	- Wetland Boundary						
	Tied Longitudinal Joint					Buried Cable Television)		
	Keyed Longitudinal Joint			вт v 3" вт v	вту вту 3"вту вту		-		
	-			REO (2")	==== BF0 (2") =====				
	Doweled Transverse Expansion Joint	E ^D ESCS B CS	Edge Of Wooded Area	BF0 2"BF0		Buried Fiber Optic			
	Doweled Transverse Contraction Joint	*	Shrubbery						
	Transverse Contraction Joint Without Dowels	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	Grove Or Orchard						
\blacksquare	Survey Reference Point	Lt. Skew Rt.	Definition Of Skew For Cross Drains						
<u> </u>	Triangulation Station		And Barrels Of Conrete Box Culverts	cas 12" cas	cas cas 12" cas	Casing			
{ B.M. NO. //2	Bench Mark	<u> </u>		от <i>4"х4"</i> от	דם דם $4''$ א דם דם דם	Duct			
	Point Of Intersection	Rt. Skew Li	t.	NPW 6" NPW	NPW NPW 6" NPW	Non Potable Water			
	North Arrow	4 4 4 4 4 4 4 4 4 A	Concrete	PET 8" PET	PET PET 8" PET	Petroleum			
- TAB - AV - MATK	Edges Of Existing Pavement And Sidewalk		Wood	RD 4" RD	RDRDRDRD 4" RDRD	Roof Drain			
	Guardrail	e	Rate Of Superelevation		sтм sтм /2" sтм	Steam			
c. c.	Crash Cushion (Attenuator)	•	Separation of Separation	SIM 12 SIM -	31101 31101 12 31101	Siedili			
	Piling Pier Column								
	Concrete Monument								
<u>B</u>	Base Line							See Genera	Note Sheet 10f 3.
<u>V</u>	Centerline								
P.	Property Line								PARTMENT OF TRANSPORTATION
	Delta Angle							RC	AD DESIGN
±	Approximate								
•	Payed Or Diameter								DD CVUDOLC

Round Or Diameter

STANDARD SYMBOLS

	Names	Dates	Approve	d By J.M	£71.
Designed By				State Roadway	Design Engineer
Drawn By	CDP	08/72	Revision	Sheet No.	Index No.
Checked By	COR	08/72	00	2 of 3	002

STANDARD SYMBOLS FOR PLAN SHEETS

TRAFFIC SIGNALS SYMBOLS LIGHTING SYMBOLS SIGNING AND PAVEMENT MARKING SYMBOLS EXISTING PROPOSED EXISTING PROPOSED (--() \bigcirc ----Traffic Signal Head (Span Wire Mounted) Pole & Luminaire Traffic Signal Head (Pedestal Mounted) (-X()Existing Pole & Luminaire To Be Removed Traffic Signal Head (Mast Arm Mounted) Final Position Of Relocated Or Adjusted Pole & Luminaire 0-0 \bigcirc Traffic Signal Pole (Concrete, Wood, Metal) High Mast Lighting Tower Vehicle Detector (Loop) X City Or Utility Owned Luminaire & Pole Signal Cable (On Messenger Wire) PVC (Polyvinyl Chloride) Lighting Conduit And Conductors Conduit Rigid Galvanized Lighting Conduit And Conductors \otimes Vehicle Detector (Points) Lighting Pull-Box Pedestrian Detector Light Distribution Point 700 - 83 Pedestrian Signal Head (Pole Or Pedestal Mounted) Joint Use Pole Controller Cabinet (Base Mounted) Pier Cap Underdeck Luminaire Controller Cabinet (Pole Mounted) \triangle Pendant Hung Underdeck Luminaire W - DW Walk - Dont Walk Flashing Dont Walk FDWSignal Face Number 5 Signal Lens Programmed Signal Head Messenger Wire

3

*(3)

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Pole Tabulation Cross Reference

Signal Phase

Pole Tabulation Cross Reference (Joint Use Pole)

See General Notes, Sheet I of 3

Pavement Arrow

Single Solid Line

Double Solid Line

Traffic Sign (Post Mounted)

Traffic Sign (Overhead)

Skip Line

Stop Bar

Sign Number

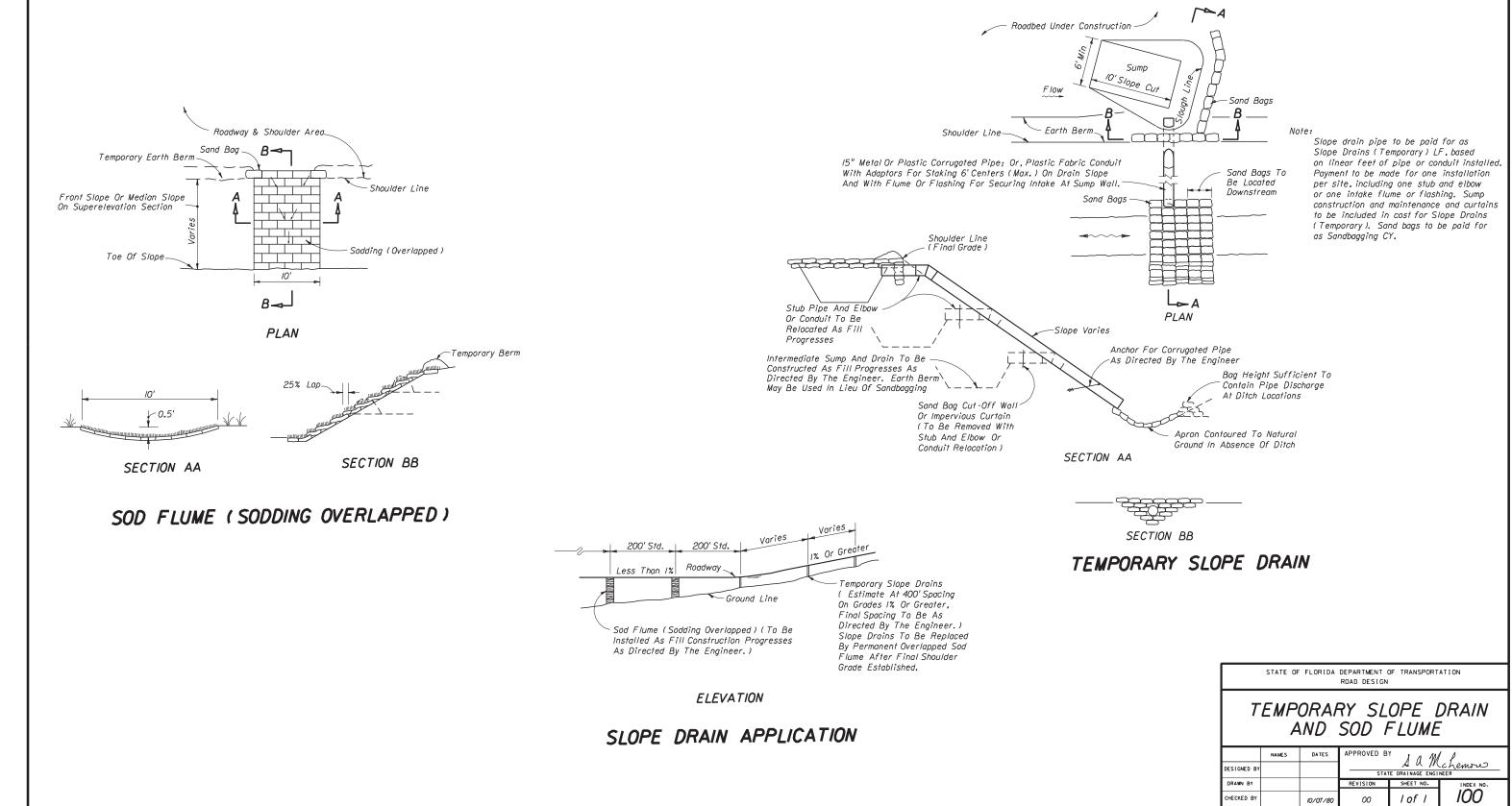
Sign Item Number

Traffic Flow Arrow

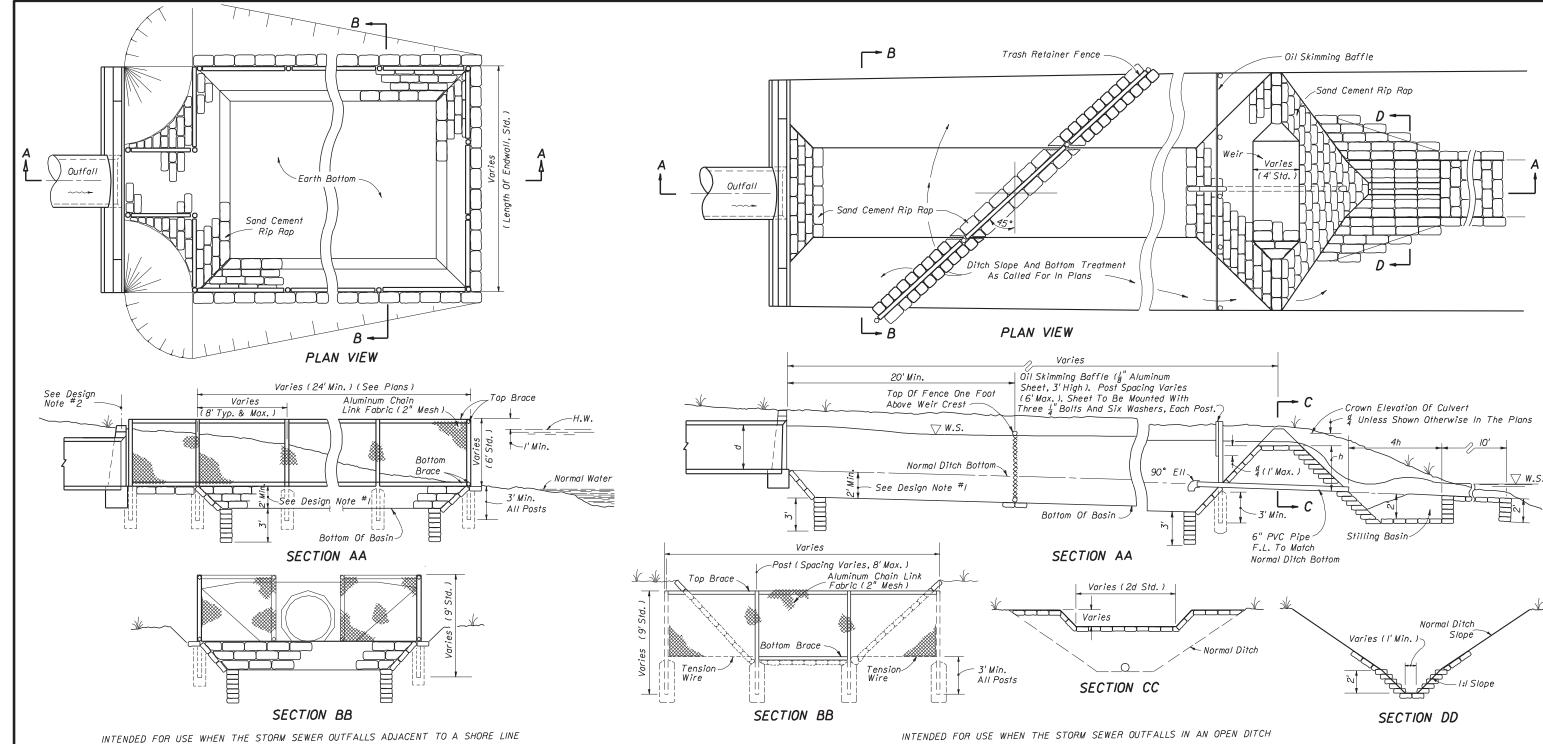
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STANDARD SYMBOLS

	Names	Dates	Approve		♪ // .
Designed By					Design Engineer
Drawn By	CDF	08/72	Revision	Sheet No.	Index No.
Checked By	COR	08/72	00	3 of 3	002



\$\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$



TYPE A

DESIGN NOTES

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- I. Basins should be as deep as practical with a minimum depth of 2.0 feet.
- 2. In Type A, when the top of endwall is below high water, fence also will be required along the top of the endwall.
- 3. In Type B, the weir shall be located as far from the endwall as practical. On steep ditch grades two or more weirs may be required. Intermediate weirs shall be constructed without stilling basins.
- 4. In Type B, the 6" PVC pipe shall be constructed unless shown otherwise in the plans.

GENERAL CONSTRUCTION NOTES

- I. Fence materials shall be aluminum or concrete only.
- 2. Aluminum posts shall be 3" diameter minimum. Aluminum rail braces shall be in accordance with Index 452. Concrete posts and rail braces shall be in accordance with Index 451. All posts to be set in concrete.
- 3. Fabric shall be installed to inside of posts and rail braces, and tied to posts and braces at 6" centers.
- 4. For additional details on fencing, see Index Nos. 451 and 452.
- 5. All basin slopes to be I: lunless detailed otherwise in the plans.
- 6. Sediment basins to be constructed prior to commencement of upland constuction. Maintenance and clean out to be by the Contractor until acceptance of project by the Engineer.

TYPE B

GENERAL NOTES

I. The cost for Type A and Type B trash retainer and sediment basins shall include the cost for riprap, fencing, baffles, piping and for sump and weir earthwork over and above ditch excavation called for in the plans. Payment for both Type A Type B shall be under the contract unit price for Sediment Basins, Each. Cleanouts as called for in the plans shall be paid for under the contract unit price for Sediment Basin Cleanouts, CO.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRASH RETAINER AND SEDIMENT BASIN

	Names	Dates	Approve	d By La M	Chemoro
Designed By	WJR	05/74		// کا لکر State Drainage I	
Drawn By			Revision	Sheet No.	Index No.
Checked By	HLB	06/74	00	I of I	101

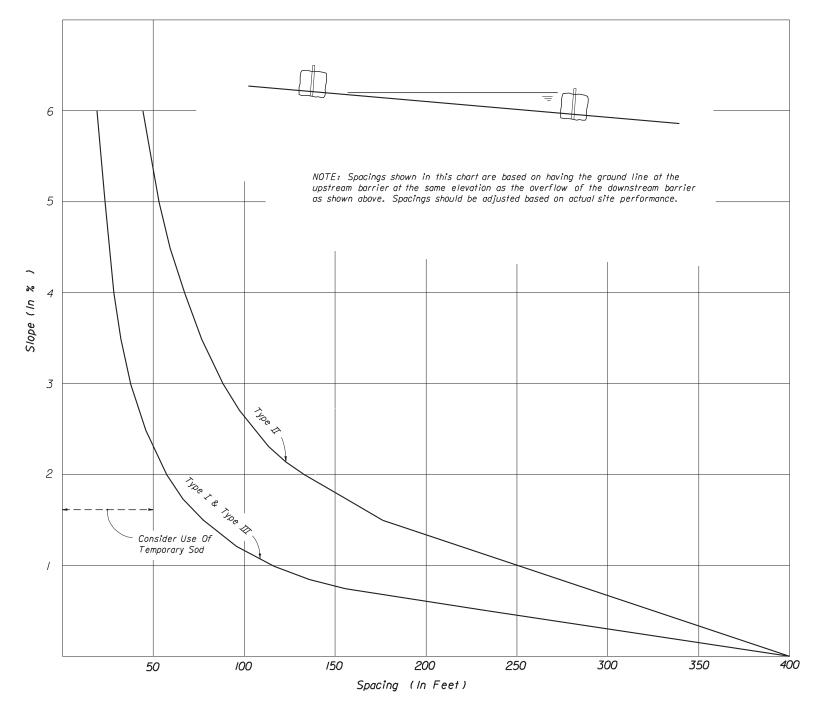
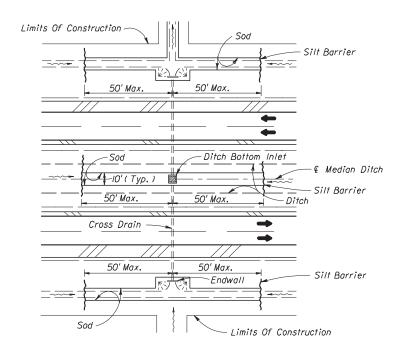


CHART I

RECOMMENDED SPACING FOR BALED HAY BARRIERS AND TYPE III SILT FENCE

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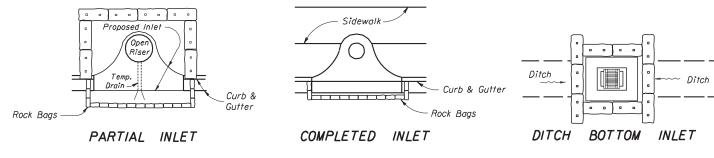


DITCH INSTALLATIONS AT DRAINAGE STRUCTURES

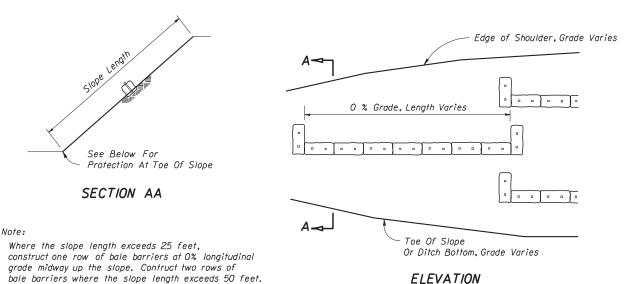
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

ROCK BAG, BALED HAY, BALED STRAW AND SILT FENCE BARRIERS

	Names	Dates	Approve	ic By I 1 M	1 0
Designed By	EGR	02/80			chemous age Engineer
Drawn By	HSD	09/82	Revision	Sheet No.	Index No.
Checked By	JVG	09/82	00	1 of 3	102



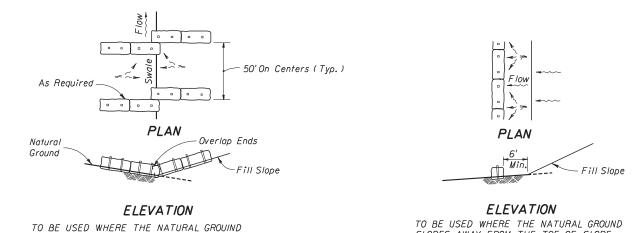
PROTECTION AROUND INLETS OR SIMILAR STRUCTURES



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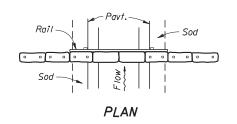
ALONG FILL SLOPE

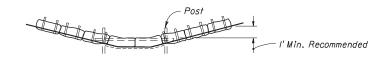


SLOPES TOWARD THE TOE OF SLOPE

AT TOE OF SLOPE BARRIERS FOR FILL SLOPES

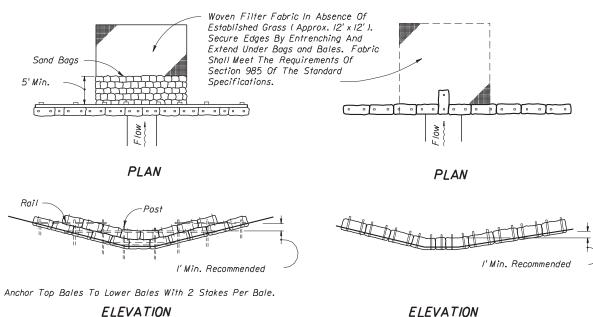
SLOPES AWAY FROM THE TOE OF SLOPE





ELEVATION

BARRIER FOR PAVED DITCH



ELEVATION

TYPE II

TYPE I

BARRIERS FOR UNPAVED DITCHES

NOTES FOR BAILED HAY OR STRAW BARRIERS

- I. Type I and ${\rm I\!I}$ Barriers should be spaced in accordance with Chart I, Sheet I.
- 2. Hay bales shall be trenched 3" to 4" and anchored with 2 1" x 2" (or 1" dia.) x 4' wood stakes. Stakes of other material or shape providing equivalent strength may be used if approved by the Engineer. Stakes other than wood shall be removed upon completion of the project.
- 3. Rails and posts shall be 2" x 4" wood. Other materials providing equivlalent strength may be used if appoved by the engineer.
- 4. Adjacent bales shall be butted firmly together. Unavoidable gaps shall be plugged with hay or straw to prevent silt from passing.
- 5. Where used in conjunction with silt fence, hay bales shall be placed on the upstream side of the fence.
- 6. Bales to be paid for under the contract unit price for Baled Hay or Straw, EA. The unit price shall include the cost of filter fabric for Type I and II Barriers. Sand bags shall be paid for under the unit price for Sandbagging, CY. Rock bags to be paid for under the contract unit price for Rock Bags, EA.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

ROCK BAG, BALED HAY, BALED STRAW AND SILT FENCE BARRIERS

	Names	Dates	Approve	d By	1/ 0
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Checked By	HLB	6/74	00	2 of 3	102

TYPE IV SILT FENCE

ELEVATION

NOTES FOR SILT FENCES

- I. Type III Silt Fence to be used at most locations. Where used in ditches, the spacing for Type III Silt fence shall be in accordance with Chart I, Sheet I.
- 2. Type IV Silt Fence to be used where large sediment loads are anticipated. Suggested use is where fill slope is 1:2 or steeper and length of slope exceeds 25 feet. Avoid use where the detained water may back into travel lanes or off the right of way.

SECTION

- 3. Do not construct silt fences across permanent flowing watercourses. Silt fences are to be at upland locations and turbidity barriers used at permanent bodies of water.
- 4. Where used as slope protection, Silt Fence is to be constructed on 0% longitudinal grade to avoid channelizing runoff along the length of the fence.
- 5. Silt Fence to be paid for under the contract unit price for Staked Silt Fence, (LF).

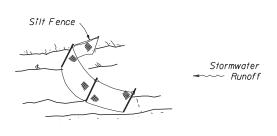
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

ROCK BAG, BALED HAY, BALED STRAW AND SILT

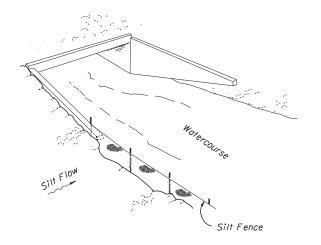
FENCE BARRIERS

Names Dates Approved By 1 1 W P

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signed By	RAA/CJA	09/85			age Engineer
awn By	LRE	09/85	Revision	Sheet No.	Index No.
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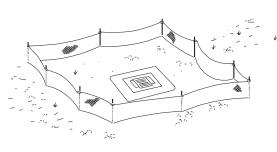


Silt Fence Protection in Ditches with Intermittent Flow



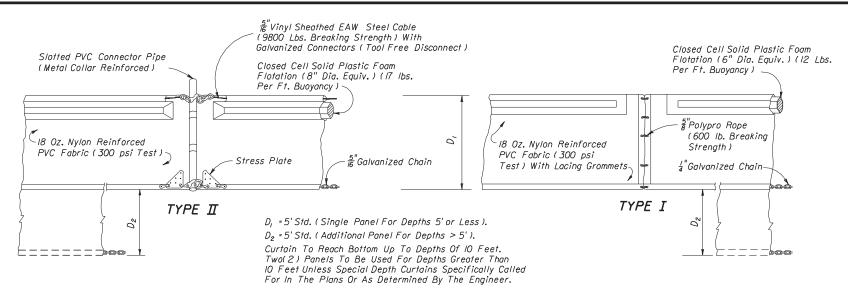
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Silt Fence Protection Around Ditch Bottom Inlets.

SILT FENCE APPLICATIONS



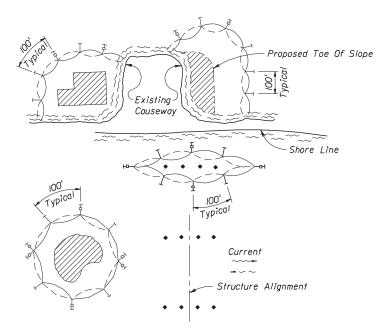
Post (Options: 2" x 4" Or 2½" Min. Dia. Wood; Steel 1.33 Lbs/Ft. Min.)

18 Oz. Nylon Reinforced PVC Fabric (300 psi Test)

STAKED TURBIDITY BARRIER

NOTICE: COMPONENTS OF TYPES I AND II MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES I AND II SHALL BE AS APPROVED BY THE ENGINEER.

FLOATING TURBIDITY BARRIERS



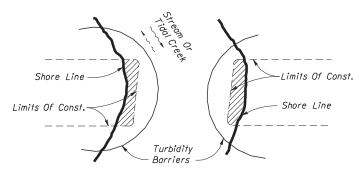
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- I. Turbidity barriers are to be used in all permanent bodies of water regardless of water depth.
- 2. Number and spacing of anchors dependent on current velocities.
- 3. Deployment of barrier around pile locations may vary to accommodate construction operations.
- 4. Navigation may require segmenting barrier during construction operations.
- 5. For additional information see Section 104 of the Standard Specifications.

LEGEND

- Pile Locations
- Dredge Or Fill Area
- —⇔ Mooring Buoy w/Anchor
- → Anchor
- Barrier Movement Due
 To Current Action



Note: Turbidity barriers for flowing streams and tidal creeks may be either floating, or staked types or any combinations of types that will suit site conditions and meet erosion control and water quality requirements. The barrier type(s) will be at the Contractors option unless otherwise specified in the plans, however payment will be under the pay item(s) established in the plans for Floating Turbidity Barrier and/or Staked Turbidity Barrier. Posts in staked turbidity barriers to be installed in vertical position unless otherwise directed by the Engineer.

TURBIDITY BARRIER APPLICATIONS

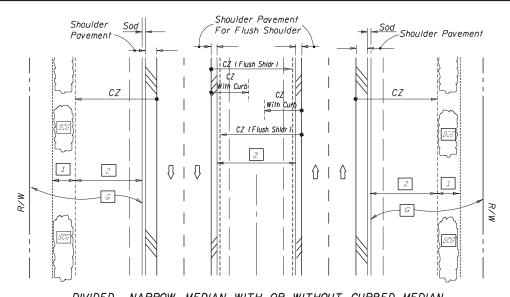
GENERAL NOTES

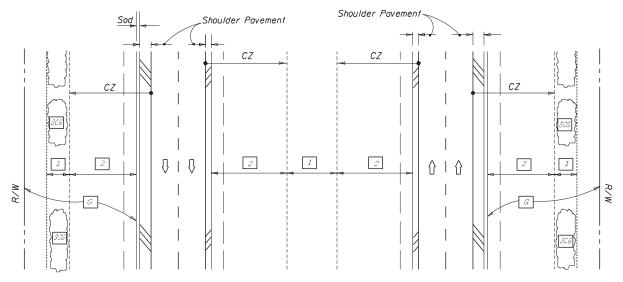
- I. Floating turbidity barriers are to be paid for under the contract unit price for Floating Turbidity Barrier, LF.
- 2. Staked turbidity barriers are to be paid for under the contract unit price for Staked Turbidity Barrier, LF.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TURBIDITY BARRIERS

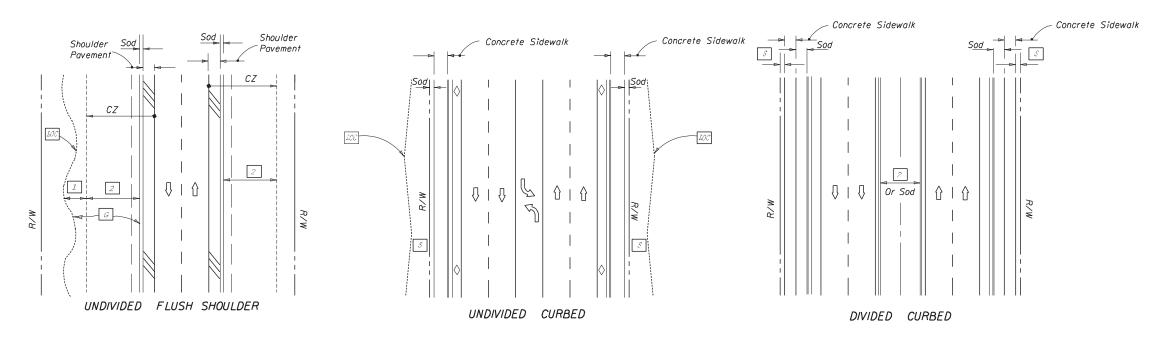
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Designed By	RAA/CJA	9/85	St	ate Drainage En	glneer
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Checked By	RAA	10/85	00	I of I	103 _





DIVIDED NARROW MEDIAN WITH OR WITHOUT CURBED MEDIAN

DIVIDED WIDE MEDIAN WITH OR WITHOUT CURBED MEDIAN



LEGEND

 \bigcirc

Wildflower Group #1

2 Wildflower Group #2

Grass-Seed/Seed & Mulch (To Limit of Construction)

Selective Clearing
And Grubbing

Limits Of Construction

Seed, Seed And Mulch, Sod Or Seed, Sod

WILDFLOWERS SEEDING RATE	
Common Name (Botanical Name)	lbs/ac
#I Group	
Black-Eyed Susan (Rudbeckia hirta)	2
Tickseed (Coreopsis tinctoria)	
Lance-Leaf Tickseed (Coreopsis lanceolata)	10
Indian Blanket (Gaillardia pulchella)	10
#2 Group	
Annual Phlox (Phlox drummondii)	10
Moss Verbena (Berbena tenuisecta)	6

		ZONE	I			ZONE	П	
	COAS	STAL	INL	AND	COASTAL		INLAND	
TYPE OF SEED	Mar. To Nov.	Nov. To Mar.	Mar. To Oct.	Oct. To Mar.	Feb. To Dec.	Dec To Feb.	Feb. To Dec.	Dec. To Feb.
PERMANENT GRASSES								
Unhulled Bermuda	20	20	20	20	20	20	20	20
Bahia Argentina Or Pensacola Bahia			80	80			80	80
QUICK GROWING GRASS								
Annual Rye		20		20		20		20
TOTAL Lbs/ PER ACRE	20	40	100	120	20	40	100	120

Note: The seeding rates shown in this table apply only when seed is spread by an approved mechanical spreader meeting the requirements of Section 570 and 577 of the Standard Specifications.

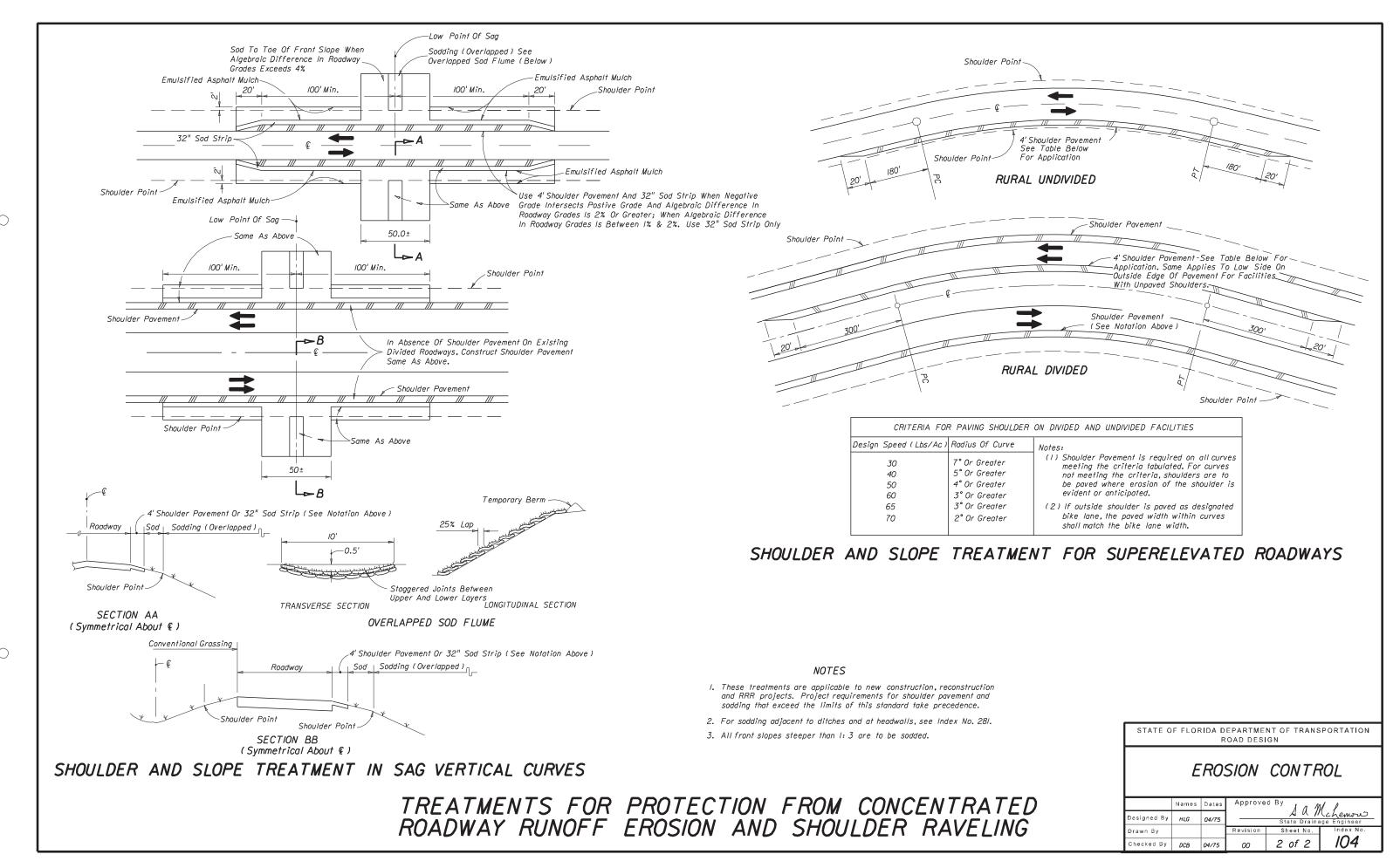
*See Index No. 105 for zone boundaries and seeding rates for shoulder reworking.

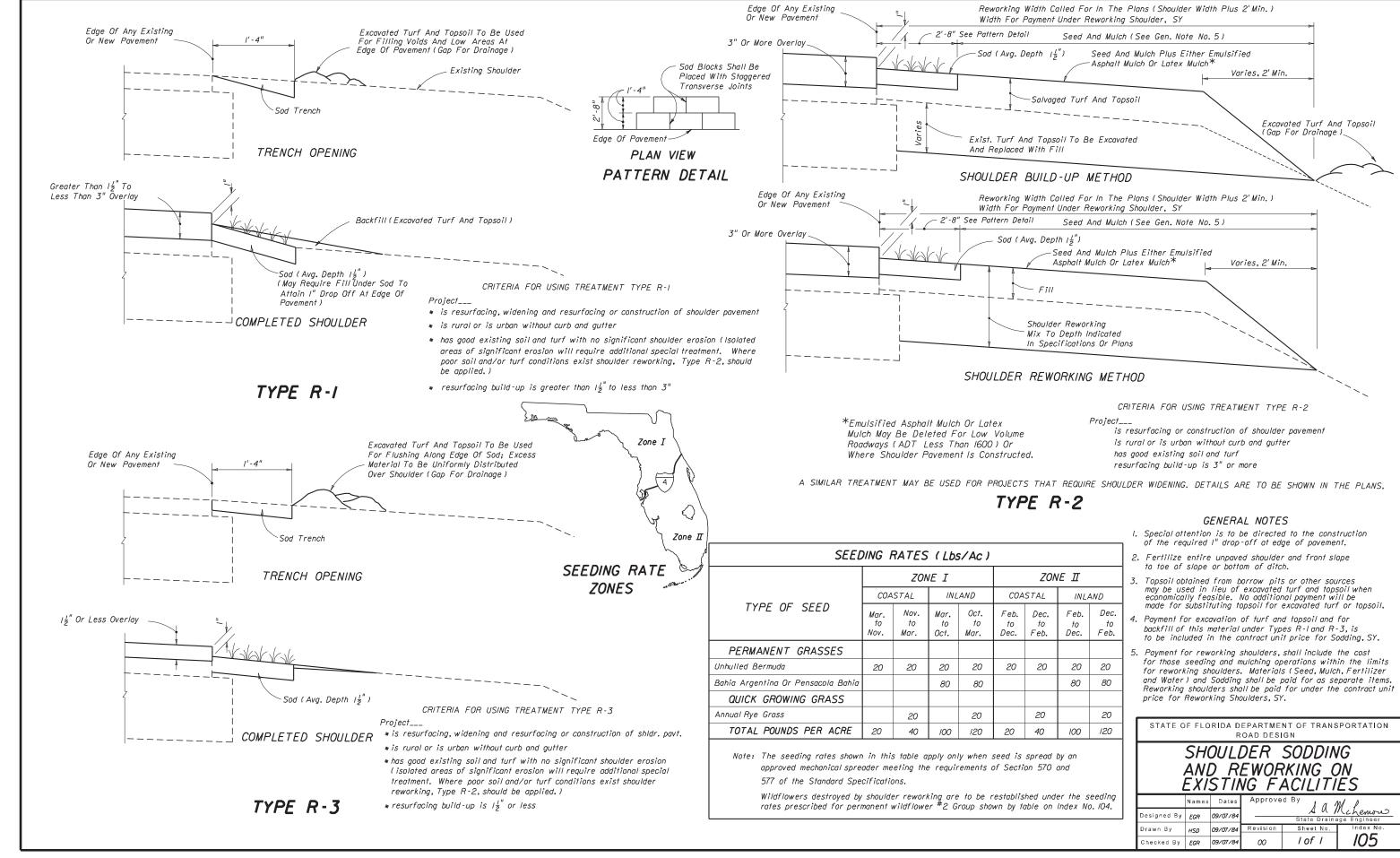
SEEDING FOR PERMANENT GRASSING AND WILDFLOWERS

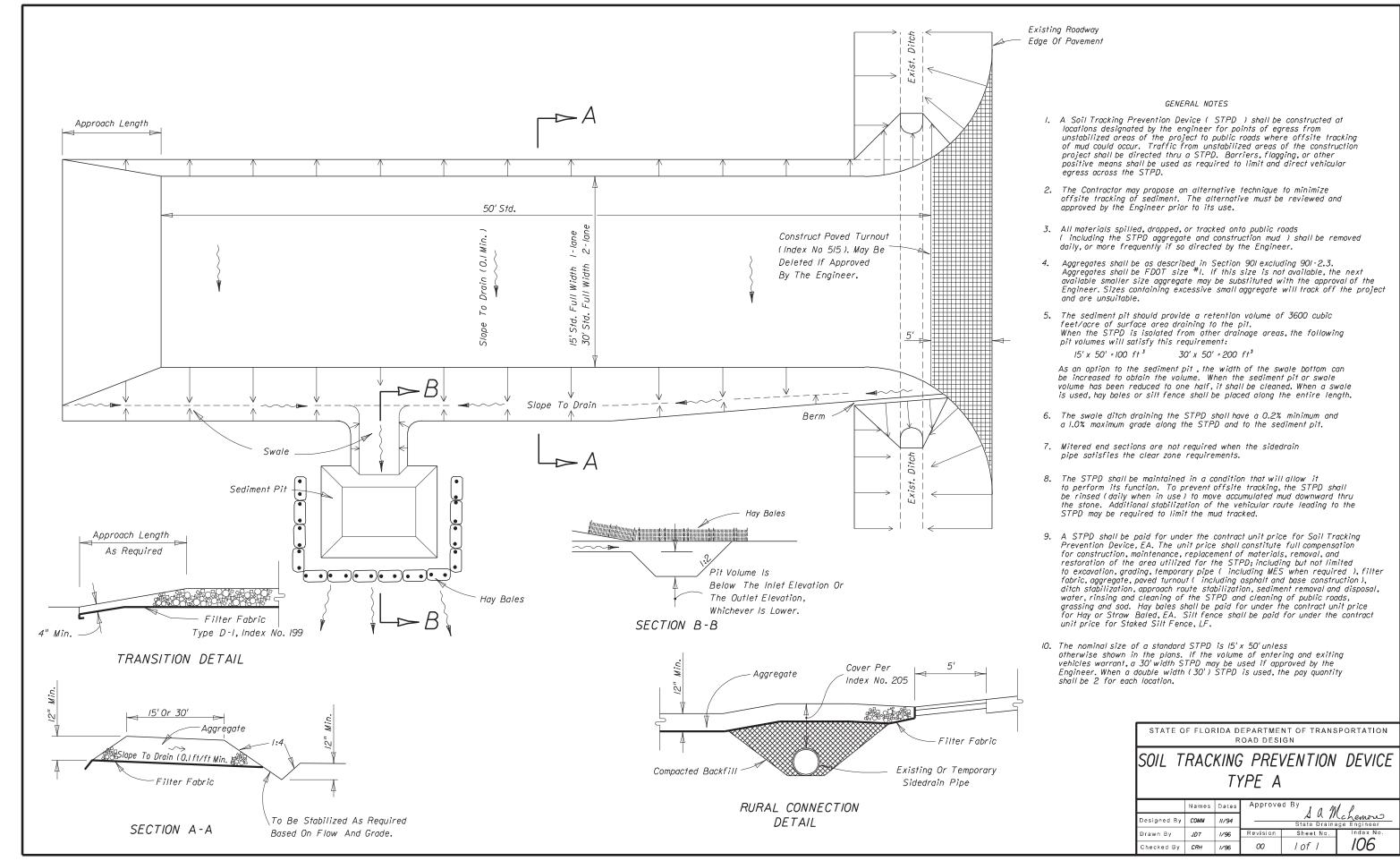
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

EROSION CONTROL

	Names	Dates	Approve	· X A W	1 1
Designed By	GJH	01/00			chemoro age Engineer
Drawn By	HSD	61/00	Revision	Sheet No.	Index No.
Checked By	GLH	01/00	00	1 of 2	104







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STANDARD CRITERIA

							0.		O E					
CLASS	1486	APPLICATION DESCRIPTION	STANDARD INDEX NO.	PERMITTIVITY SEC ^{-/}	1	0. S. EVE #	GRAB TENSILE STRENGTH	SEWN STRENGTH	PUNCTURE	TRAPEZOIDAL TEAR	WIDE WIDTH TENSILE STRENGTH	RESI	V STANCE Allowed)	COMMENTS
			, wo.				N	N	N	N	KN/M	%	Time (Hrs.)	
	D-1	Revetment (Special)		(See D-2)	(Sec	e D-2)	1400	1260	500	500		50	500	Woven Monofilament Geotextiles only (Elongation < 50%) Provide 6" thick aggregate bedding layer.
		Revetment (Standard) Articulating Block			% SOIL F No. 200 S		Woven Monofilament	Woven Monofilament	Woven Monofilament 400	Woven Monofilament 250				Woven Geotextiles only. No Slit Film Geotextiles allowed. Provide I50 mm thick aggregate bedding layer for revetment (standard). The bedding layer may be omitted if a D-I fabric is used with revetment (standard).
	D-2	Gabions Rock, Rubble, Broken Concrete	281		2 15% to 50 > 50%	70 *	Other Geotextiles: Elongation < 50% 1400 > 50% 900	Other Geotextiles: Elongation < 50% 1200 > 50% 810	Other Geotextiles: Elongation < 50% 500 > 50% 350	Other Geotextiles: Elongation < 50% 500 > 50% 350		50	500	*For cohesive soils with a plasticity index > 7, maximum average role value for AOS is number 50 sieve.
		Underdrain * * *	286	% SOIL PASSING No. 2000 SIEVE	% SOIL P No. 2000		Elongation	Elongation	Elongation	Elongation				No woven slit film fabrics allowed.
DRAINAGE	D-3	French Drain Sheet Piling Filter	285	∠ 15% 0.5 15% to 50% 0.2	5 < 15% 2 15% to 50		< 50% 1100 > 50% 700	< 50% 990 > 50% 630	< 50% 400 > 50% 250	∠ 50% 400 **		50	500	*For cohesive soils with a plasticity index > 7, maximum average role value for AOS is number 50 sieve.
(D)		Filter Fabric Jacket (Culvert) Concrete Pavement Subdrainage	280 287		./ > 50%	70 *								** Required Trapezoidal tear for woven monofilament is 250. *** See Index No. 286 for the permittivity and AOS values of the internal filter fabric of type V underdrain.
	D-4	Slope Pavement (Sand-Cement) Ditch Pavement (Sand-Cement)	281	0.	5	40	800	720	220	/55		50	500	Nonwoven only. Min. Thickness 90 Mils Elongation ≥ 50%
	D-5	Mechanical Stabilized Retaining Wall Cast-In-Place Retaining Wall		0.		40	400	360	220	175		50	500	
	D-6	Slope Pavement (Concrete) Ditch Pavement (Concrete)	281	0.:	5	40	800	720	220	/55		50	500	Nonwoven only. Min. Thickness I2O Mils Elongation \geq 50%
EROSION	E -1	Staked Silt Fence	102	0.5	5	NA	400	360	NA	/55		80	500	Minimum Filtration Efficiency of 75% and minimum flow rate of 0.3 gal.
(E)	E-2	Wind Screen		0.5	5	NA	400	360	NA	NA		80	150	
	E-3	Plastic Erosion Mat (Turf Reinforcement Mat) (Type I)	NA	NA		NA	NA	NA	NA	NA	15 x 11	80	2,000	Maximum Permissible design velocity 3.0 M/Sec
	E - 4	Plastic Erosion Mat (Turf Reinforcement Mat) (Type 2)	NA	NA		NA	NA	NA	NA	NA	29 x 21	80	2,000	Maximum Permissible design velocity 4.3 M/Sec
	E-5	Plastic Erosion Mat (Turf Reinforcement Mat) (Type 3)	NA	NA		NA	NA	NA	NA	NA	44 x 32	80	2,000	Maximum Permissible design velocity 5.5 M/Sec
STABILIZATION	R-I	Reinforcement		0.0	5	30	880	800	400	400		80	150	
(R)	R-2	Separation		0.0	5	30	800	720	355	220		_	_	

(I) Type refers to FDOT class and application.

TABLE I

Test Permittivity AOS	Unit sec -/ mm	Test Method ASTM-D-4491 ASTM-D-4751
Elongation Grab Tensile Strength	% N	ASTM-D-4632 ASTM-D-4632
Wide With Tensile Strength Maximum Design Velocity	kN/M M/sec	ASTM-D-4595 See Design Note 3
Sewn Strength Puncture	N N	ASTM-D-4884 ASTM-D-4833
Trapezoidal Tear Ultra Violet Resistance	N % Retained	ASTM-D-4533 ASTM-D-4355
Filtration Efficiency	In Strength %	ASTM-D-5141
Flow Rate	L ³ /min.	ASTM-D-5141

GENERAL NOTES

- I. Specifications for geotextiles are in Section 985. Physical criteria for each application is provided by this standard, in conjunction with those sections.
- All values except AOS are MINIMUM AVERAGE ROLL values in the weakest principal direction. Values for AOS are MAXIMUM AVERAGE ROLL values.
- 3. Test soil or fill material adjacent to the geotextile for gradation to select values for permittivity and AOS.
- Unless specifically restricted in COMMENTS column, any type of material may be used.

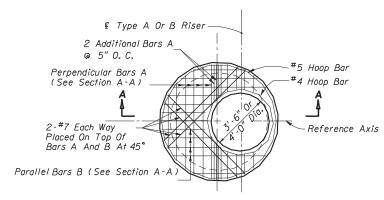
DESIGN NOTES

- The Designer shall review this criteria and adjust the values as necessary to satisfy project requirements. These adjustments shall be called for in the plans or contained in the project special provisions.
- 2. U V Resistance: The value represents the percent of minimum textile strength retained (ASTM-D-4632) after weathering per ASTM-D-4355 for the test period (hours).
- 3. Maximum design velocity for plastic erosion mats shall be determined by tests performed by Utah State University, Texas Transportation Institute or an independent testing laboratory approved by the State Drainage Engineer.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

GEOTEXTILE CRITERIA

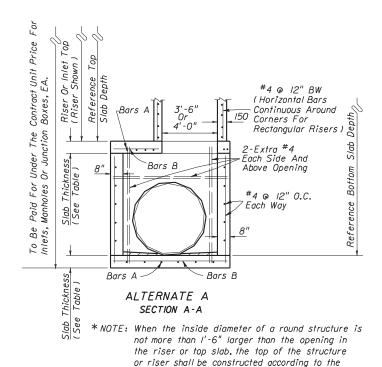
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esigned By	CONN	8/9/	Sta	chemore Ineer	
rawn By	DLD	8/91	Revision	Sheet No.	Index No.
hecked By	КНН	8/91	00	I of I	199



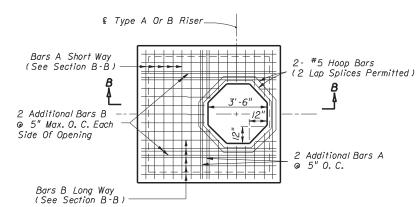
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Note: Not Applicable For Type C, D & E Ditch Bottom Inlets. See Index No. 232.

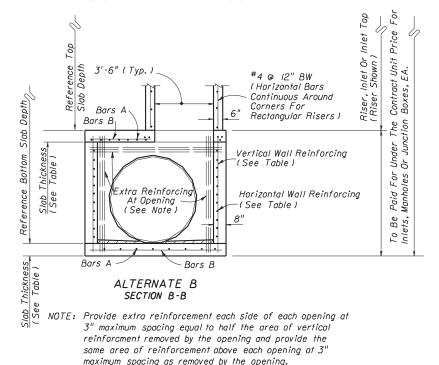
TOP SLAB REINFORCING STEEL DIAGRAM

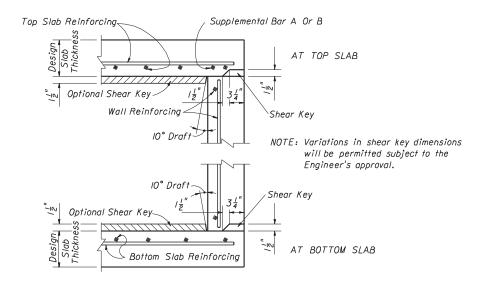


"Special Top Slab" details on this sheet.

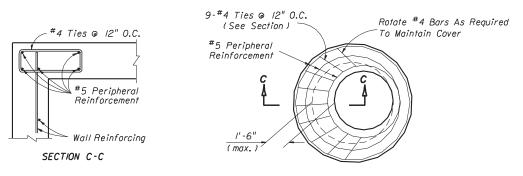


TOP SLAB REINFORCING STEEL DIAGRAM





SLAB TO WALL DETAILS FOR PRECAST ALTERNATE WITH 8" WALLS



SPECIAL TOP SLAB *

GENERAL NOTES

- Standard structure bottoms 4'-0" diameter and smaller (Alt. A) and 3'-6" square (Alt. B) are designated Type P. Larger standard structure bottoms are designated Type J. Risers are permitted for all structures.
- 2. Walls of circular structures (Alternate A) constructed in place may be of non-reinforced concrete or brick or reinforced concrete. Precast and rectangular structures (Alternate B) shall be constructed of reinforced concrete only.
- 3. Wall thickness and reinforcement are for either reinforced cast-in-place or precast concrete units except that precast circular units may be furnished with walls in accordance with either A.S.T.M. C478 (up to 96' diameter) or A.S.T.M. C76. Class III, B Wall, modified where the elliptical steel cage area is placed in the center one-third of the wall.
- 4. Top and floor slab thickness and reinforcement are for precast and cast in place construction. Top and floor slabs shall be of Class II concrete. concrete. Concrete as specified in A.S.T.M. C478 (4000 psi) may be used in lieu of Class I and Class II concrete in precast items manufactured in plants which are under the 'Standard Operating Procedures' for the inspection of precast drainage products.
- 5. All reinforcement shown is A.S.T.M. A6/5/A6/5M Grade 60 steel, either smooth or deformed. Equivalent area Grade 40 steel or Grade 65KS/ welded wire fabric may be substituted according to index No. 20/.

- 6. Structure bottoms may be used in conjunction with curb inlet tops Types 1, 2, 3, 4, 5, 6, 9, and 10, and any manhole or junction box unless otherwise shown in the plans or other standard drawings. Alt. B structure bottoms may be used in conjunction with curb inlet Types 7 & 8, or any ditch bottom inlet unless otherwise shown in the plans or other standard drawings.
- 7. Rectangular structures may be rotated as directed by the Engineer in order to facilitate connections between the structure walls and storm sewer pipes.
- 8. Except when ACI hooks are specifically required, reinforcement top and slab shall be straight embedment.
- 9. All steel bars shall have $l_{z}^{\frac{1}{2}m}$ minimum cover unless otherwise shown except for precast circular units manufactured under ASTM C76 or ASTM C478. Horizontal steel in rectangular structures shall be lapped a minimum of 24 bar diameters at corners.

- 10. The corner fillets shown are necessary for rectangular structures used with circular risers and inlet throats and used on skew with rectangular risers, inlet and inlet throats. Fillets will be required in lieu of the bottom slab of the Alt. B riser when used with the Alt. A box. Each fillet shall be reinforced with 2-#5 bars.
- II. Inlet throats, riser or manhole tops shall be secured to structures as shown on Index No. 201.
- 12. Structures with depths over 14' are to be checked for floatation by designer of project drainage.
- I3. Units larger than specified standard may be substituted at the contractor's option when these units will not cause or increase the severity of utility conflicts. Such larger units shall be furnished at no additional cost to the Department. Larger Alternate A units cannot replace Alternate B units without approval of the Engineer. This note applies to this Index only.
- For manhole and junction box tops, for frames and covers, and, for supplementary details see Index No. 201.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

STRUCTURE BOTTOMS TYPE J AND P

	Names	Dates	Approve	. 1 /1 /11	1 P
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Drawn By			Revision	Sheet No.	Index No.
Checked By			00	1 of 2	200

SLAB DESIGNS - SQUARE AND RECTANGULAR STRUCTURES

(ALL SLABS 8" THICK - REINFORCING PARALLEL TO SHORT WAY AND LONG WAY)

SH0RT	-WAY	LONG - WAY			
SLAB DEPTH	SCHEDULE	SLAB DEPTH	SCHEDULE		
Si	ZE: 3'-6" × UNLIMITE	D	<u>'</u>		
≥ 0.5′ < 29′	В	≥ 0.5′ - 40′	В		
29' - 40'	С				
Si	ZE: 4'-O" × UNLIMITE	ED .			
≥0.5′<19′	В	≥0.5' < 34'	В		
19' <29' 29' -40'	C D	34' - 40'	С		
Si	ZE: 5' x 5'				
≥0.5′<3′		≥0.5'<3'	С		
3' <19'	B	3' <19'	B		
19' <28'	C	19' <28'	C		
28' <38'	D	28' <38'	D		
38' - 40'	F	38' - 40'	F		
	'ZE: 5' × 6'				
≥0.5'<3'	С	≥ 0.5′ < 3′	С		
3' <16'	В	3' <20'	В		
16' <24'	С	20' <29'	С		
24' <34' 34'-40'	D F	29' - 40'	D		
	ZE: 5' x 7'				
		> 0 51 + 71	1 0		
≥0.5' < 3' 3' 4'</td <td>C B</td> <td>≥0.5' < 3' 3' <22'</td> <td>C B</td>	C B	≥0.5' < 3' 3' <22'	C B		
14' <21'	C	22' < 33'	C		
21' < 39'	D	33'-40'	D D		
39' - 40'	F	33 ,3			
SI	 ZE: 5' × 8'				
≥0.5′ < 3′	С	≥ 0.5′ < 39′	В		
3' <8'	В	39' - 4 0'	С		
8' <17'	С				
17' <23'	D				
23' -40'	F				
	ZE: 5' x 9'				
≥0.5′ < 3′	С	≥ 0.5' < 32'	В		
3' <8'	В	32'-40'	С		
8' <17'	С				
17' <23'	D				
23' - 40'	F				

SHORT	-WAY	LONG - WAY			
SLAB DEPTH	SCHEDULE	SLAB DEPTH	SCHEDULE		
SIZ	?E: 6' × 6'		•		
≥0.5′<3′	D	≥ 0.5′ < 3′	D		
3' <4'	C	3' <4'	C		
4' < 4'	В	4' 4'</td <td>В</td>	В		
14' < 21'	С	14' <21'	С		
21' <28'	D	21' <28'	D		
28' - 40'	F	28' - 40'	F		
	?E: 6' x 7'		· · · · · · · · · · · · · · · · · · ·		
≥0.5′<3′	D	≥0.5′<3′	D		
3' <4'	C	3' <4'	C		
4' <12'	B	4' 5'</td <td>В</td>	В		
12' < 19'	C	15' < 21'	C		
19' <26'	D	21' < 30'	D		
26'-40'	F	30'-40'	F		
	! ?E: 6' × 8'	30 -40	1		
		>0 E! + 7!	1 0		
≥ 0.5′ < 3′ 3′ <4′	D	≥ 0.5′ < 3′	D C		
	C	3' <4'			
4' < 7'	В	4' < 16'	В		
7' <16'	С	16' <23'	С		
16' <23'	D	23' < 32'	D		
23'-40'	F	32'-40'	F		
SIZ	?E: 6' x 9'				
≥0.5′<3′	D	≥ 0.5′ < 3′	D		
3' 5'</td <td>С</td> <td>3' <4'</td> <td>С</td>	С	3' <4'	С		
15' <21'	D	4' < 18'	В		
21' < 27'	Ε	18' < 27'	С		
27' - 40'	G	27' <37'	D		
		37' - 40'	E		
SIZ	?E: 7' x 7'		•		
≥0.5′<3′	E	≥0.5′<3′	Ε		
3' <4'	D	3' <4'	D		
4' < 16'	C	4' < 16'	C		
16' <22'	D	16' <22'	D		
22' < 28'	E	22' < 28'	E		
28'-40'	G	28' - 40'	G		
	?E: 7' × 8'	20 10			
		>0 E' < 3'	E		
≥ 0.5' < 3' 3' <4'	E	≥ 0.5′ < 3′ 3′ < 4′	<i>E D</i>		
	D				
4' < 15'	C	4' < 17'	С		
15' < 21'	D E	17' <23'	D		
21' < 27'		23' < 29'	E		
27'-40'	G	29' - 40'	G		
	?E: 7' x 9'				
≥0.5′<3′	Ε	≥ 0.5′ < 3′	E		
3' <4'	D	3' <4'	D		
4' < 12'	С	4' < 18'	С		
4' < 12' 12' < 18'	D	18' < 24'	D		
4' < 12'		4' < 18' 18' < 24' 24' < 32' 32' - 40'			

REINFORCING SCHEDULE

I. Slab reinforcement is appropriate for top, intermediate, and bottom slabs.

GENERAL NOTES

- 2. Slab depth is measured from finished grade to top of slab.
- 3. Wall design depth is measured to the top of the bottom slab for boxes and to the top of the intermediate slab for risers.
- 4. Wall height is the distance between top of lower slab to bottom of upper slab.
- 5. Wall sizes exceeding 9'-0" require a special design.

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SHORT	-WAY	LONG - WAY			
SLAB DEPTH	SCHEDULE	SLAB DEPTH	SCHEDULE		
SI	ZE: 6' × 6'				
≥0.5′<3′	D	≥0.5′<3′	D		
3' <4'	C	3' <4'	C		
4' 4'</td <td>В</td> <td>4' <!--4'</td--><td>В</td></td>	В	4' 4'</td <td>В</td>	В		
14' < 21'	C	14' <21'	C		
21' < 28'	D	21' < 28'	D		
28' - 40'	F	28'-40'	F		
		20 -40	T F		
≥0.5′<3′	D	≥0.5′<3′	D		
3' <4'	С	3' <4'	С		
4' 2'</td <td>В</td> <td>4' <!--5'</td--><td>В</td></td>	В	4' 5'</td <td>В</td>	В		
12' < 19'	С	<i>15' < 21'</i>	С		
<i>19' <26'</i>	D	21' <30'	D		
26' - 40'	F	30'-40'	F		
	ZE: 6' × 8'				
≥0.5′<3′	D	≥ 0.5′ < 3′	D		
3' <4'	C	3' <4'	C		
4' < 7'	В	4' < 16'	В		
7' <16'	С		C		
		16' <23'			
16' <23'	D	23' < 32'	D		
23'-40'	F	32'-40'	F		
_	ZE: 6' × 9'				
≥0.5′<3′	D	≥0.5′<3′	D		
3' <15'	С	3' <4'	С		
<i>15' <21'</i>	D	4' < 18'	В		
21' < 27'	E	18' <27'	С		
27' - 40'	G	27' <37'	D		
		37' - 40'	E		
SI	ZE: 7' x 7'				
≥0.5′<3′	E	≥0.5′<3′	E		
3' <4'	D	3' <4'	D		
4' < 16'	C	4' < 16'	C		
16' < 22'	D	16' <22'	D		
22' < 28'	E	22' < 28'	E		
22 < 20 28' - 40'	G	22 < 28 28' - 40'	G		
		20 70	1 0		
≥0.5' < 3'	E	≥ 0.5′ < 3′	E		
≥ 0.5 < 3 3' < 4'	D	≥ 0.5 < 3 3' <4'	D E		
			C		
4' < 15'	С	4' < 17'			
15' < 21'	D	17' <23'	D		
21' < 27'	E	23' < 29'	E		
27' - 40'	G	29' - 40'	G		
	ZE: 7' x 9'				
≥0.5′<3′	Ε	≥0.5′ < 3′	E		
3' <4'	D	3' <4'	D		
4' < 12'	С	4' < 18'	С		
12' < 18'	D	18' <24'	D		
18' < 24'	Ε	24' <32'	Ε		
24'-40'	G	32'-40'	G		

SCHEDULE	GRADE 60 STEEL OR 65 KSI(WIRE FABRIC) in ² /ft		
Α	0.20		
В	0.24		
С	0.37		
D	0.53		
Ε	0.73		
F	1.06		
G	1.45		

SLAB DESIGNS - ROUND STRUCTURES

SHURT-WAY		LONG -	WAY						
SLAB DEPTH	SCHEDULE	SLAB DEPTH	SCHEDULE	SLAB DEPTH	SLAB THICKNESS	REINFORCING (2 WAYS) SCHEDULE			
SIA	ZE: 8' × 8'				SIZE: 3'-6"				
≥ 0.5' <3'	D	≥ 0.5′ <3′	D	≥ 0.5′-40′	8"	С			
3' < 4'	C	3' < 4'	C		-				
4' < 9'	В	4' < 9'	В						
9' <17'	C	9' <17'	C						
17' <31'	D	17' <31'	D						
31'-40'	G	31'-40'	G						
	ZE: 8' × 9'	31 10			SIZE: 4'-0"				
≥ 0.5' <3'	D	≥ 0.5' <3'	E	≥ 0,5'-40'	8"	С			
3' < 4'	C	3' < 4'	D	20.5 -40	U	C			
4' < 16'	В	4' < 18'	C						
16' < 22'	C	18' < 25'	D						
22' <29'	D		F F						
		25' <32' 32'-40'	· ·						
29'-40'	F	32 -40	G		0.75 5/ 0"				
	ZE: 9' x 9'	1 > 2 = 1			SIZE: 5'-0"				
≥ 0.5' <3'	F	≥ 0.5′ <3′	F	≥0.5' < 30'	8"	C			
3' < 14'	С	3' < 14'	С	30' - 40'	8"	D			
14' <20'	D	14' <20'	D						
20' <26'	Ε	20' <26'	E						
26' - 40'	G	26'-40'	G						
VERTICAL	REINFORCING	HORIZONTAL	REINFORCING	8' < 18' 18' < 30' 30' < 37'	8" 8" 8"	С D Е			
				37'-40'	8"				
WALL	SCHEDULE	WALL	SCHEDULE	37 - 40	σ	G			
DEPTH	JOHEDOLL	DEPTH	301120022						
					SIZE: 8'-0"				
	ZE: 3'-6" * SEE			≥ 0.5′ < 9′	10"	С			
≥1.17′-40′	Α	≥ 1.17′ - 40′	В	9' <15'	10"	D			
				15' <23'	10"	E			
SI	ZE: 4'-0"	<u> </u>		23' < 33'	12"	Ε			
≥1.17'-40'	A A	≥ 1,17'-40'	В	33'-40'	12"	G			
- 1111 10		10			SIZE: 10'-0"				
SIA	ZE: 5'-0"			≥0.5′ <6′	10"	С			
≥1,17'-40'	Α	≥1,17' <33'	В	6' < 11'	10"	<i>D</i>			
10	† ''	33'-40'	C		10"				
<u>~.</u>	75 61 61	1 00 .0		17' <23'	12"	E			
SI	ZE: 6'-0"			23'-40'	12"	<u>Е</u> G			
≥ 1.17′-40′	Α	≥ 1.17' <22'	В	25 - 40	12	<u> </u>			
		22'-40'	С		SIZE: 12'-0"				
SI	ZE: 7'-0"			≥0.5′ <6′	12"	С			
≥1,17'-40'	A	≥1.17′<15′	В	6' < //'	12"	<u>с</u> Д			
· -		15' <25'	C	11' < 16'	12"	υ E			
		25'-40'	D	16' <20'	14"				
	25 0! 0!!	•			14"	E			
SI	ZE: 8'-0"			20' - 40'	14"	G			

SIZE is the inside length of a structure wall.

SHORT - WAY

SIZE: 9'-0"

≥ 1.17′ - 40′

≥1.17′-40′

LONG - WAY

≥ /,/7' <//' 19' <29'

29'-40'

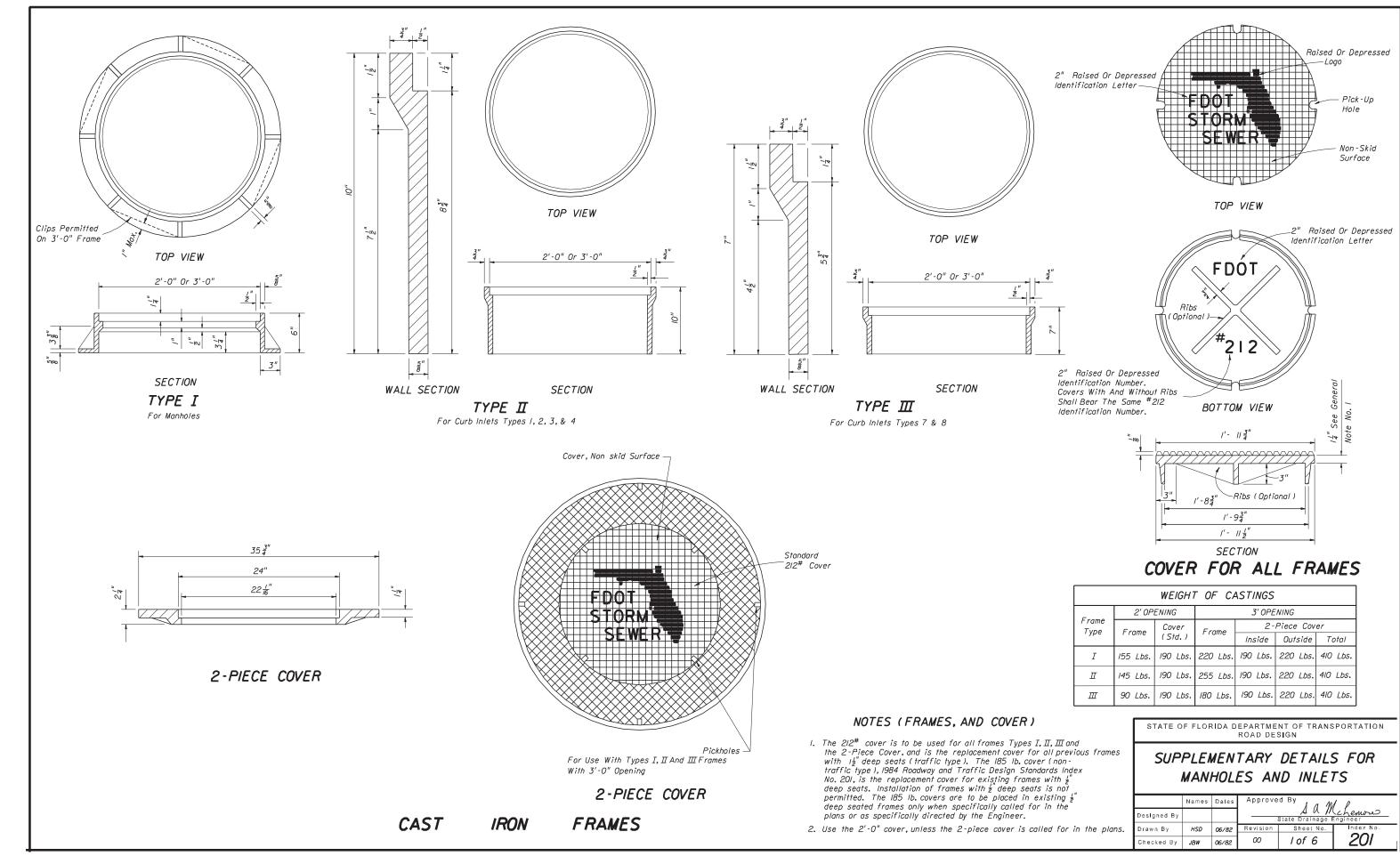
≥1.17' <15' 15' <22'

* Precast structures 3'-6" x 3'-6" maybe cast with 6" walls to depths of 15'. See Index 201

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

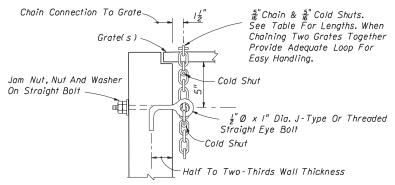
STRUCTURE BOTTOMS TYPE J AND P

	Names	Dates	Approve	1 /1 /1//	P		
Designed By			State Drainage Engineer				
Drawn By	dds	05/86	Revision	Sheet No.	Index No.		
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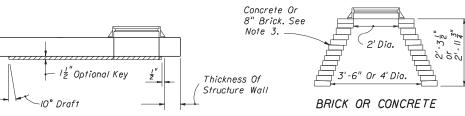
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Note: When Alternate G grate is specified, the chain, bolt, nuts, washer and cold shuts shall be galvanized in accordance with the specifications for the grate.

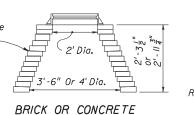
> Cost of eye bolt and chain to be included in the contract unit price for inlets.

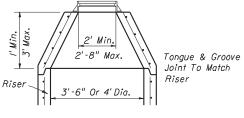
	EYE BOLT AND CHAIN REQUIREMENTS									
Index Number	Inlet Type	Eye Bolts	Length Of Chain	Handling & Remarks						
217	(MB)	1	4'-0"	Slide & Spin						
	(MB)2	1	4'-0"	Slide & Spin						
	(MB)3	2	2@4'-0"	Slide & Spin						
	(MB) 4	2	2 @ 4'-0"	Slide & Spin						
	(MB)5	2	2 @ 4'-0"	Slide & Spin						
218	(BW)	1	3'-8"	Slide Or Slide & Spin						
219	(BW,RGD)	1	4'-0"	Slide & Spin						
220	S	1	4'-0"	Slide & Spin						
221	V	1	4'-0"	Slide & Spin						
230	Α	1	3'-0"	Slide						
231	В	1	5'-0"	Slide & Spin						
232	С	1	2'-6"	Slide & Spin						
	D	1	2'-6"	Slide & Spin						
	Ε	2	2 @ 2'-6"	Slide & Spin						
	Н	2	2 @ 2'-6"	Flip Ctr. Grate and Slide & Spin Single Free Grate						
			/@ /'-6"	Ctr. Grate To One End Grate						
233	F	1	3'-6"	Flip Or Slide & Spin						
	G	/	6'-0"	Slide						
			2'-0"	Lifting Loop						
234	J	1	4'-0"	Slide & Spin						



SECTION Note: See Slab Designs Index 200.

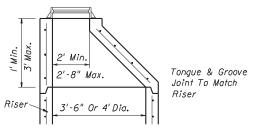
TYPE 7





PRECAST CONCENTRIC CONE

TYPE 8



PRECAST ECCENTRIC CONE

TOPS MANHOLE

NOTES (TOPS)

- I. Manhole top Type 7 slabs shall be of Class II concrete. Concrete as specified in ASTM C478 may be used for precast units; see General Note No. 3.
- 2. Manhole top Type 7 slabs may be of cast-in-place or precast construction. The optional key is for precast tops and in lieu of dowels. Frame and slab openings are to be omitted when top is used over a junction box. Frames can be adjusted with from one to six courses of brick.
- 3. Manhole top Type 8 may be of cast-in-place or precast concrete construction or brick construction. For concrete construction, the concrete and steel reinforcement shall be the same as the supporting wall unit. An eccentric cone may be used.
- 4. Manhole tops shall be secured to structures by optional construction joints as shown on Sheet 3 of 6.
- 5. Substitution of manhole top Type 8 for manhole top Type 7 is allowed provided that minimum dimensions shown above are not reduced.

DESIGN NOTES

I. Manhole top Type 8 should be specified in the plans when depths shown above can be maintained.

Typical Location For Bottom Slab Without Sump Dia. Varies (12" Std.) 4" Galvanized Hardware Cloth No. 4 Coarse Aggregate 2' x 2' x 2' Filter Fabric

NOTE: Sump bottom appropriate for all manhole and inlet types. Cost for sump bottom to be included in the contract unit price for inlet or manhole.

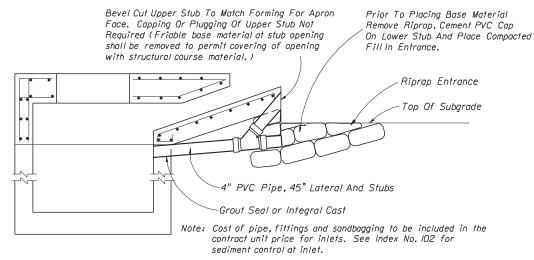
SUMP BOTTOM

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SUPPLEMENTARY DETAILS FOR MANHOLES AND INLETS

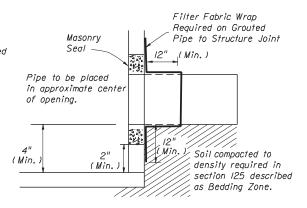
	Names	Dates	Approve	1/1/1	M P			
Designed By	HLB	04/75	State Drainage Engineer					
Drawn By			Revision	Sheet No.	Index No.			
Checked By	LMF	04/75	00	2 of 6	201			

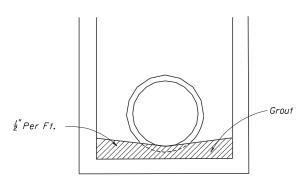
EYE BOLT AND CHAIN FOR LOCKING GRATES TO INLETS



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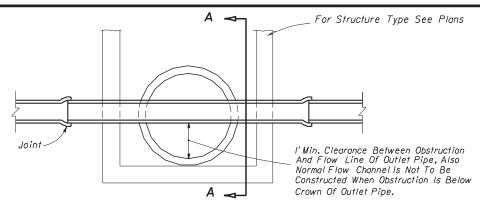
TEMPORARY DRAINS FOR SUBGRADE AND BASE





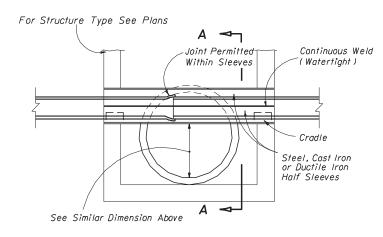
Note: Grout to consist of 3: I Sand-Cement Mixture or any Class Concrete. FOR ALL STRUCTURES UNLESS EXCLUDED BY SPECIAL DETAIL

ALL PIPE TYPES DRAINAGE STRUCTURE INVERT



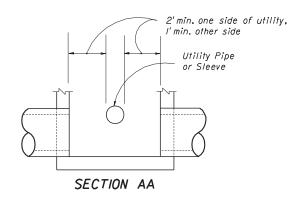
NOTE: No joints allowed inside the Condition I structure.

CONDITION I



NOTE: Only water mains will be allowed to pass through a Condition ${\it I\!I}$ structure.

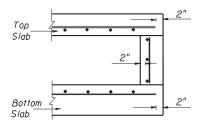
CONDITION II



DESIGNERS NOTE

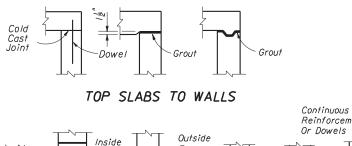
"Sumped" conflict manholes shall not be used unless the system is hydraulically designed to take in account the headloss generated if the sump is completely blocked.

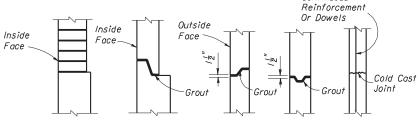
UTILITY PIPES THRU STORM SEWER STRUCTURES

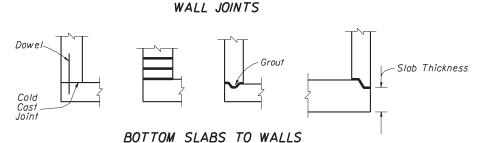


(NOTE: NOT APPLICABLE AROUND MANHOLE AND RISER OPENINGS)

REBAR STRAIGHT END EMBEDMENT FOR TOP AND BOTTOM SLABS





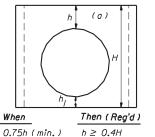


I. One or more types of joints may be used in a single structure, except

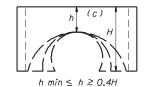
brick wall structure. Brick wall construction is permitted on circular units only.

- 2. All grouted joints are to have a maximum thickness of I".
- 3. Keyways are to be a minimum of l_2^{l} deep.
- 4. Joint dowels are to be #4 bars, I2" long with a minimum of 6 bars per joint approximately evenly spaced for circular structures or 2 bars per side at approximate quarter points for rectangular. Bars are to be placed approximately 6" into fresh concrete leaving the remainder to extend into the secondary cast. Welded wire fabric may be substituted for the dowels bar in accordance with the equivalent steel area table on Index 201, Sheet 4.
- 5. Minimum cover on reinforcing bars is $l_2^{l''}$.
- 6. Joints between wall segments and between wall segments and top or bottom slabs may be sealed either by preformed plastic gasket material using the procedures given in Section 430-7.3 or by grout.
- 7. Approved product inserts may be used in lieu of dowel embedment.

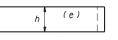
OPTIONAL CONSTRUCTION JOINTS



 $h_1 < 0.75h \ (min.)$ $h \ge 0.4H$ $h_1 \ge 0.75h \ (min.)$ $h \ge h \ (min.)$



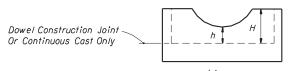
Segments may be inverted. Maximum opening for pipe shall be the pipe 0.D. plus 6". If h can not be attained, then a top or bottom slab must be attached to the segment as shown below.



 $h \ge h$ (min.)

ĺ	Mini	num Value For	h
I	h (min.)	Box Or Riser D	iameter
I	/'-O"	1'-0" & 4'	-0"
ı	/'-6"	5'-0" & 6	'-0"
ı	2'-0"	>6'-0"	

SEPARATE RISER SEGMENTS WITH CONSTRUCTION JOINTS OTHER THAN DOWEL OPTION



(h min Tabulated Above Do Not Apply)

TOP OR BOTTOM SEGMENT FOR DOWEL CONSTRUCTION JOINTS OR CONTINUOUS CAST SEGMENTS COMPARATIVE SIDE VIEWS MINIMUM DIMENSIONS FOR BOX AND RISER SEGMENTS

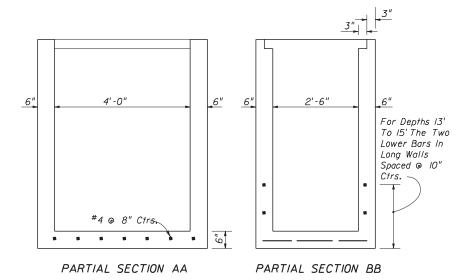
GENERAL NOTES

- I. For square or rectangular precast drainage structures, either deformed or smooth welded wire fabric may be used provided:
 - a) The smooth welded wire fabric shall comply with ASTM Al85, and deformed welded wire fabric shall comply with ASTM A497.
 - b) Width and length of the unit is four times the spacing of the cross wires.
 - c) Wire fabric shall be continuous around the box, spliced at quarter point(s) with overlap of not less than the spacing of the cross wires plus 2".
- 2. For equivalent steel areas for precast drainage structures, see Sheet 4.
- 3. Horizontal steel in the walls of rectangular structures shall be lapped a minimum of 24 bar diameter at corners.
- 4. Welding of splices and laps is permitted. The requirements and restrictions placed on welding in AASHTO M259 shall apply.
- 5. Rebar straight end embedment or peripheral reinforcement may be used in lieu of ACI standard hooks for top and bottom slabs except when hooks are specifically called for in plans or standard drawings.
- 6. Concrete as specified in ASTM C478, (4000 psi) may be used in lieu of Class I and Class II concrete in precast items manufactured in plants which are under the 'Standard Operating Procedures For The Inspection Of Precast Drainage Products'.
- 7. Maximum opening for pipe shall be the pipe o.d. plus 6". Mortor used to seal the pipe into the opening will be of such a mix that shrinkage will not cause leakage into or out of the structure.
- 8. For pay item purposes, the height used to determine if a drainage structure is less than or greater than IO feet shall be computed using (a) the elevation of the top of the manhole lid, (b) the grate elevation or the theoretical gutter grade elevation of an inlet, or (c) the outside top elevation of a junction box less the flow line elevation of the lowest pipe or to top of sump floor.

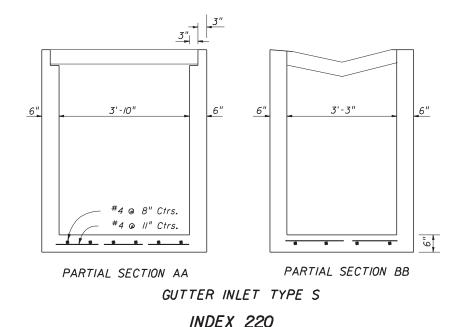
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

SUPPLEMENTARY DETAILS FOR MANHOLES AND INLETS

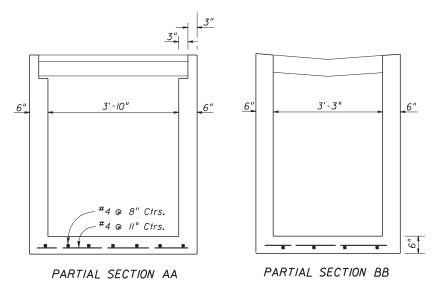
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Drawn By			Revision	Sheet No.	Index No.			
Checked By	LMF	04/75	00	3 of 6	201			



DITCH BOTTOM INLET TYPE F
INDEX 233



INDEX 231



GUTTER INLET TYPE V

AND DITCH BOTTOM INLET TYPE J

INDEX 221 & 234

NOTES FOR THIN-WALL PRECAST OPTIONS

- The details on Sheets 4,5 & 6 are optional for precast inlet construction up to depths of 15'.
 These inlets can be used with Alt. "B" Bottoms, Index 200. Cast-in-place construction must adhere to the details contained on the referenced indexes.
- 2. Only the dimensions and reinforcement changes or other modifications are indicated. For all other dimensions and details, the referenced index drawings apply. When these precast units are used inconjunction with Alt "B" Structure Bottoms, Index 200, the interior dimensions of an Alt. "B" Bottom can be adjusted to reflect these inlet interior dimensions.
- 3. Concrete which meets the requirements of ASTM C478 shall be used for structures constructed to these details.
- 4. Reinforcement can be either deformed bar reinforcement or welded wire fabric. Bar reinforcement other than 40 ksi may be used, however only two grades are recognized; Grade 40 and Grade 60. Welded wire fabric, including deformed welded wire fabric, will be recognized as having a design strength of 65 ksi. The area of reinforcement required may be reduced in accordance with the Equivalent Steel Area Table provided. For bars and spacings not given, the steel area required can be determined by the following equations:

Grade 60 Steel Area = A_s 60 = $\frac{40}{60}$ x A_s 40 Welded Wire Fabric Steel Area = A_s 65 = $\frac{40}{65}$ x A_s 40

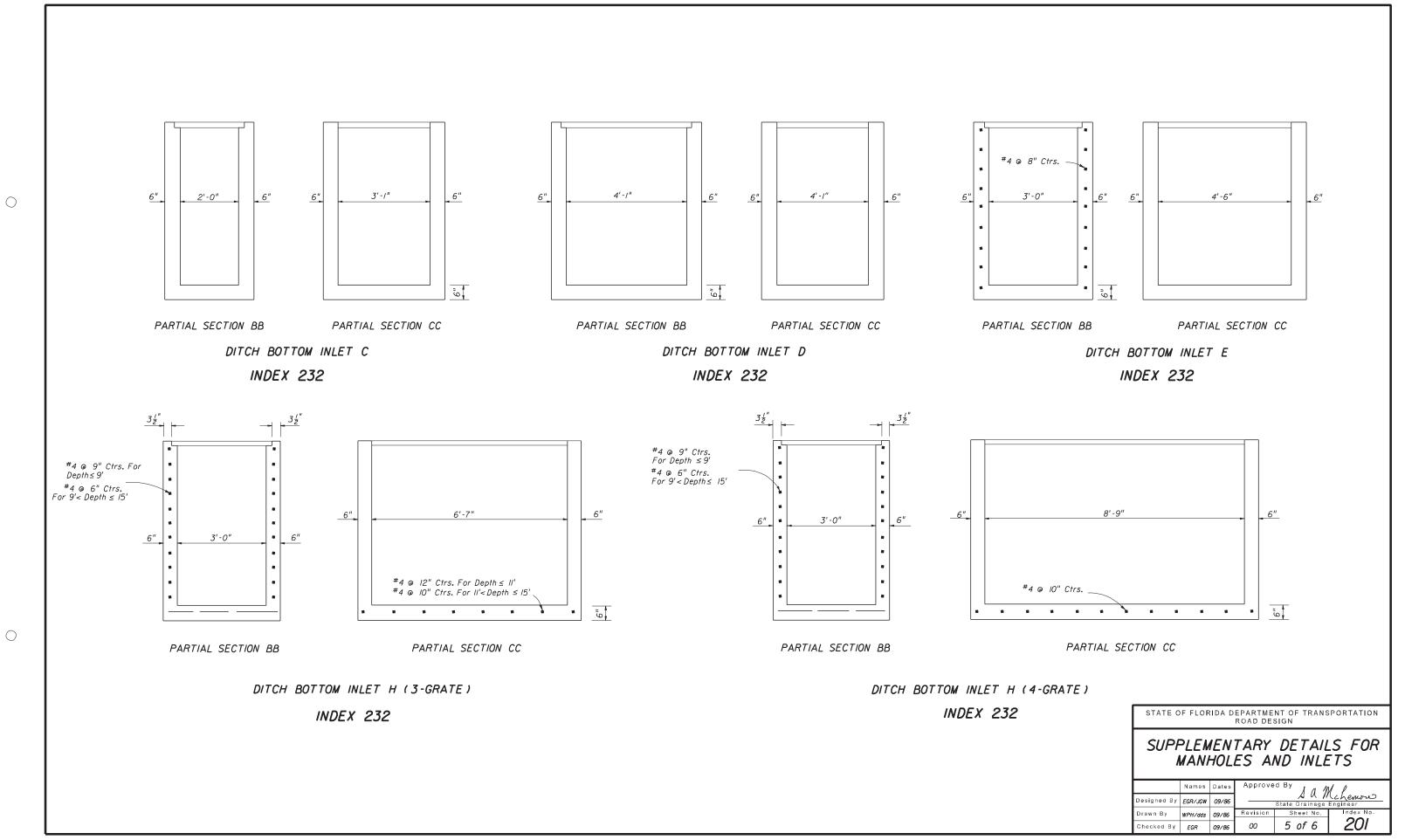
In no case will fabric with wires smaller than W3.1 or spacings greater than 8" be permitted. Bar reinforcement shall show the minimum yield designation grade mark of either the number 60 or one (I) grade mark line to be acceptable at the higher value. Maximum bar spacing shall not be greater than two (2) times the slab thickness with a maximum spacing of 12" or three (3) times the wall thickness, with a maximum spacing of 18".

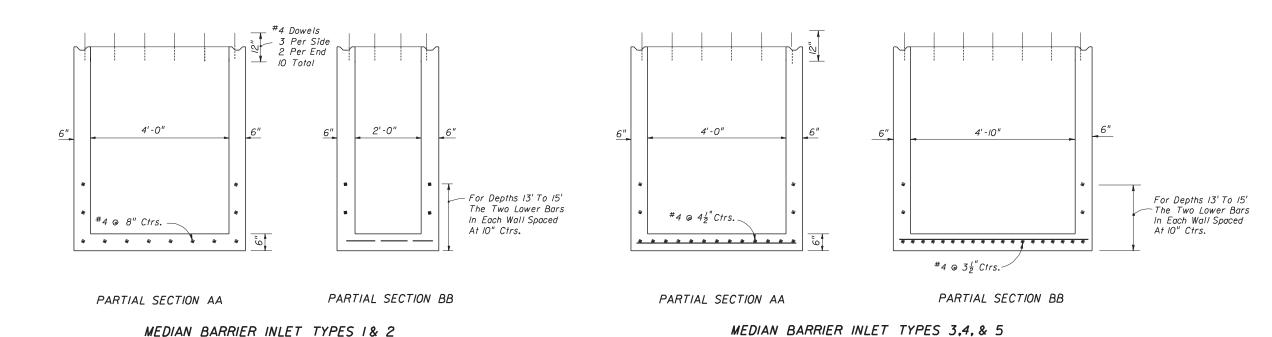
	EQU	JIVALENT ST	TEEL ARE	A TABLE		
GRADE 4 REINFORCING		EQUIVALENT REINFORCIN		EQUIVALENT 65 Ksi WELDED WIRE FABRIC		
Bar Size & Steel Spacing Area		Bar Size & Spacing	Min. Steel Area	Style Designation	Min. Steel Area	
#4 @ I2" CCEW	0.20	#3 @ 9½ ["] CCEW	./333	3" x 3" - W3.1 x W3.1 or 4" x 4" - W4.5 x W4.5 or 6" x 6" - W6.5 x W6.5	.1230	
#4 @ 9" CCEW	0.267	#4 @ 13 ½" CCEW or #3 @ 7" CCEW	.1778	3" x 3" - W4.5 x W4.5 or 4" x 4" - W5.5 x W5.5 or 6" x 6" - W8.5 x W8.5	.1641	
#6 @ 6" CCEW	0.88	#5 @ 6" CCEW or #6 @ 9" CCEW	.5867	4" x 4" - W20 x W20 or 6" x 6" - W30 x W30	.54/5	
#7 @ 6" CCEW	1.20	#6 @ 6½"CCEW or #7 @ 9" CCEW	.80	4" x 4" - W26 x W26	.7385	

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SUPPLEMENTARY DETAILS FOR MANHOLES AND INLETS

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Drawn By	WPH/dds	09/86	Revision	Sheet No.	Index No.
Checked By	EGR	09/86	00	4 of 6	201





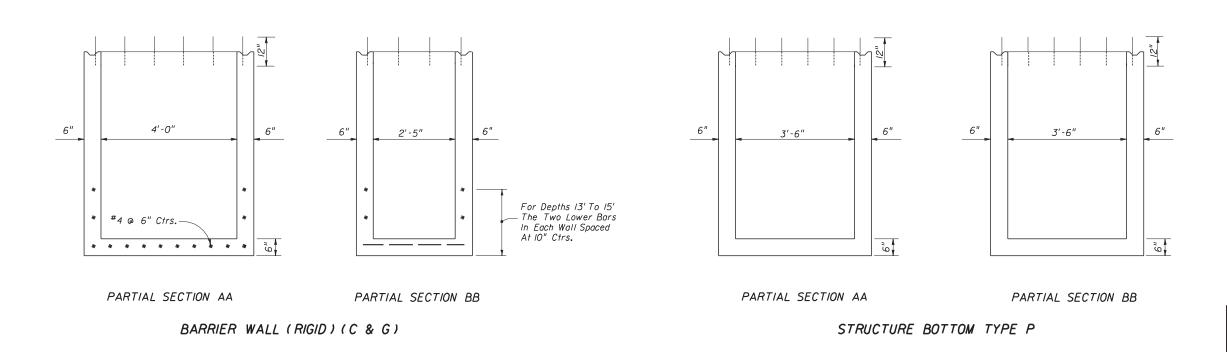
INDEX 217

MEDIAN BARRIER INLET TYPES 1 & 2

INDEX 219

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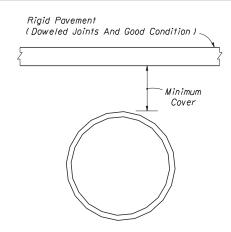


INDEX 200

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

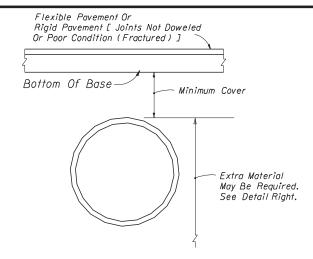
SUPPLEMENTARY DETAILS FOR MANHOLES AND INLETS

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Drawn By			Revision	Sheet No.	Index No.
Checked By			00	6 of 6	201



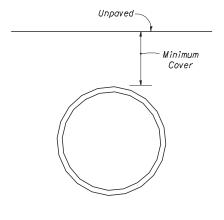
RIGID PAVEMENT

THOID TAVEMENT	
PIPE TYPE/SIZE & SHAPE	MINIMUM COVER
CONCRETE (See Note 6)	
Round & Elliptical	6"
CORRUGATED STEEL	
15"-72" Round & Arch Equiv.	9"
78" & Larger Round & Arch Eq.	15"
CORRUGATED ALUMINUM	
15"-72" Round & Arch Equiv.	9"
78"-102" Round & Arch Equiv.	15"
108" & Larger Round	18"
CORRUGATED POLYETHYLENE	
15"-48" Round	9"
POLYVINYL CHLORIDE	
15" -48" Round	9"



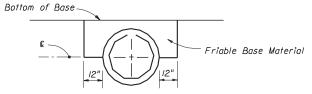
FLEXIBLE PAVEMENT

PIPE TYPE/SIZE & SHAPE	MINIMUM COVER
CONCRETE (See Note 6)	
Round & Elliptical	6"
CORRUGATED STEEL	
12"-30" Round	12" [12"]
36"-48" Round	18" (12") [15"]
54" -72" Round	21" (15") [18"]
78"-96" Round	(18") [27"]
102" & Larger Round	(24") [33"]
15"-30" Arch Equivalent	I8" [I8"]
36"-48" Arch Equivalent	24" (12") [18"]
54"-72" Arch Equivalent	27" (15") [24"]
78"-96" Arch Equivalent	(18") [30"]
IO2" & Larger Arch Equivalent	(24")
CORRUGATED ALUMINUM	
12"-24" Round	I5" [I2"]
30" - 48" Round	18" (12") [18"]
54"-72" Round	24" (18") [24"]
78" - 102" Round	(24")[30"]
108" & Larger	(30")
15"-24" Arch Equivalent	24" [21"]
30"-48" Arch Equivalent	27" (15") [24"]
54"-72" Arch Equivalent	30" (18") [27"]
78"-90" Arch Equivalent	(24")[30"]
96"-102" Arch Equivalent	(30")
CORRUGATED POLYETHYLENE	
15" - 48" Round	15"
POLYVINYL CHLORIDE	
/5" -48" Round	15"



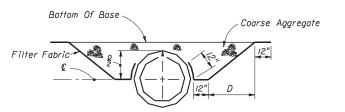
UNPAVED

		IMUM VER
PIPE TYPE/SIZE & SHAPE	COMMERCIAL	NON - COMMERCIAL
CONCRETE (See Note 6)		
Round & Elliptical	9"	3"
CORRUGATED STEEL		
12" - 30" Round	I8" [I5"]	12" [12"]
36"-48" Round	18" (12") [15"]	12" (12") [12"]
54"-72" Round	18" (12") [15"]	15" (12") [12"]
78"-96" Round	(18") [27"]	(12") [12"]
102" & Larger Round	24" [33"]	I8" [2I"]
15"-30" Arch Equivalent	18" [18"]	12" [12"]
36"-48" Arch Equivalent	24" (12") [21"]	18" (12") [15"]
54"-72" Arch Equivalent	30" (18") [24"]	24" (12") [18"]
78"-96" Arch Equivalent	(24")[27"]	(18") [21"]
102" & Larger Arch Equivalent	(30")	(24")
CORRUGATED ALUMINUM		
12"-24" Round	21" [21"]	I5" [I5"]
30" -48" Round	24" (18") [21"]	18" (12") [15"]
54"-72" Round	30" (24") [27"]	24" (18") [21"]
78" - 102" Round	(30") [33"]	(24") [27"]
108" & Larger	36"	30"
15"-24" Arch Equivalent	27" [24"]	24" [21"]
30"-48" Arch Equivalent	33" (21") [27"]	27" (15") [21"]
54"-72" Arch Equivalent	36" (24") [30"]	30" (18") [24"]
78"-90" Arch Equivalent	(30") [36"]	(24")[30"]
96"-102" Arch Equivalent	(36")	(30")
CORRUGATED POLYETHYLENE		
15" -48" Round	21"	/5"
POLYVINYL CHLORIDE		
15" - 48" Round	21"	/5"



The cost of furnishing and installing the extra base material shall be included in the cost of the culvert.

FRIABLE BASE



The coarse aggregate shall be placed in 6 inch lifts and compacted sufficiently as to be firm and unyielding. The coarse aggregate shall be gravel or stone meeting the requirements of Section 901-2 or 901-3 respectively. The gradation shall meet Section 901-6, Grades 4, 467, 5, 56, or 57 unless restricted in the plans. The filter fabric shall be Type D-3 (See Index 199). The cost of furnishing and installing the coarse aggregate and filter fabric shall be included in the cost of the culvert.

ASPHALTIC CONCRETE BASE

Note: Extra material is required when cross culverts are located on facilities subject to high speed traffic (≥55 mph) or high traffic volumes (>1600 ADT) and the cover is less than 12 inches For Concrete Pipe, 15 inches For Corrugated Steel Pipe And 18 inches For Corrugated Aluminum Pipe, Corrugated Polyethylene And Corrugated Polyvinyl Chloride Pipe.

EXTRA MATERIAL FOR CROSS CULVERTS UNDER FLEXIBLE PAVEMENTS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

COVER HEIGHT

	Names	Dates	Approve	· 111 W	1 0
Designed By	RGR	09/84			chemore age Engineer
Drawn By	DAE	09/84	Revision	Sheet No.	Index No.
Checked By	EGR	09/84	00	lof 5	205

GENERAL NOTES

- I. The tabulated values are recommended minimum dimensions to withstand anticipated highway traffic loads. Additional cover may be required to support construction equipment loads or highway traffic loads before pavement is completed. Some size thickness combinations may require minimum cover greater than those listed above. See Sheets 2, 3, & 4.
- 2. Less than the tabulated minimum cover may be used provided suitable method (s) are detailed in the plans.
- 3. Values shown in parentheses () are for 3" x l" corrugations which must be specified to utilize the lesser cover.
- 4. The tabulated values in the brackets [] apply to Type I-R (Spiral Rib) pipe which must be specifed to utilize the lesser cover.

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- 5. Commercial and noncommercial refers to typical vehicular utilization of unpaved roads and drives where rutting and cover displacement may occur.
- 6. For Pipe Class S with diameters of 12" to 30", the minimum height of fill measured from top of finished grade to outside top of pipe is 3 feet.

MINIMUM COVER FOR CONCRETE, STEEL, ALUMINUM, POLYETHYLENE AND POLYVINYL CHLORIDE PIPE

ROUND PIPE DIMENSIONS Wall Thickness (In.)* Equiv. Classes II, III, IV, V Dia. Area A WALL B WALL C WALL (Sq. Ft.) (In.) 12 0.8 15 1.2 24 NA 18 1.8 NA 24 3./ 3 3 3 30 4.9 $2\frac{3}{4}$ $4\frac{1}{4}$ 36 7./ 3 4 $4\frac{3}{4}$ 42 9.6 3 ½ 4 ½ 5 ½ 48 $5\frac{3}{4}$ 12.6 4 5 54 15.9 $4\frac{1}{2}$ 5½ 6<u>1</u> $6\frac{3}{4}$ 60 19.6 6 23.8 $5\frac{1}{2}$ 7 🕹 6 / 72 $7\frac{3}{4}$ 28.3 6 78 33.2 7 🚽 81/4 84 38.5 8 $8\frac{3}{4}$ 90 44.4 8 ½ 94 96 50.3 8 9 $9\frac{3}{4}$ 56.7 102 $9\frac{1}{2}$ 10 4 63.7 108 9 10 $10\frac{3}{4}$ 114 70.9

10

120

78.5

ELLIPTICAL PIPE DIMENSIONS										
Nom	inal D	imens	sions			Wall				
Horiz.		Ve	rt.	Equiv.		Thickness (In.)				
Rise (In.)	Span (In.)		Span (In.)	Dia.	Area (Sq.Ft.)	Classes HE II, III, IV VE II, III, IV				
NA	NA	NA	NA	12	NA	NA				
12	18	18	12	15	1.3	2½				
14	23	23	14	18	1.8	2 3				
19	30	30	19	24	3.3	3 ¼				
24	38	38	24	30	5./	3 3				
29	45	45	29	36	7.4	4 ½				
34	53	53	34	42	10.2	5				
<i>38</i>	60	60	38	48	12.9	5 <u>/</u>				
43	68	68	43	54	16.6	6				
48	76	76	48	60	20.5	6 <u>∤</u>				
53	83	83	53	66	24.8	7				
58	91	91	58	72	29.5	7 <u>/</u>				
63	98	98	63	78	<i>34</i> .6	8				
68	106	106	68	84	40.1	8 <u>/</u> 2				
72	113	113	72	90	46.1	9				
77	121	121	77	96	52.4	9 <u>/</u> 2				
82	128	128	82	102	59,2	10				

For	Informational Purposes	Only	

114

120

66.4

74.0

82.0

10 %

//

11 ½

136

143

151

92

97

136 | 87

92

97

143

151

ROUND PIPE INSTALLATIONS								
	Ма	Maximum Height of Fill(ft)						
PIPE DIAMETER	Class S	Class I	Class II	Class III	Class IV	Class		
12" - 30"	9	13	17	24	36	55		
36"-54"	8	12	16	22	34	52		
60" -78"	7	//	<i>1</i> 5	21	33	5/		
84"-96"	6	10	14	20	32	49		
Pipe Class S D-Load=650 Lbs/Ft/Ft(.Ol" Crack) D-Load=950 Lbs/Ft/Ft(Ultimate)								
Pipe Class I	D - Load D - Load			F†(.01" ′F†(UI†)		
Pipe Class II				/Ft(.01 /Ft(UIt)		
Pipe Class III D-Load=1350 Lbs/Ft/Ft(.OI" Crack) D-Load=2100 Lbs/Ft/Ft(Ultimate)								
Pipe Class IV D-Load=2100 Lbs/Ft/Ft(.Ol" Crack) D-Load=3150 Lbs/Ft/Ft(Ultimate)								
Pipe Class ▼				/F†(.0/ /F†(U/				

Note: At the option of the pipe supplier or the contractor, a
Pipe Class with greater strength may be substituted for
the Pipe Class designated in the plans.

ELLIPTICAL PIPE INSTALLATIONS (AII Sizes)

Installation	Maximum Height Of Fill (Ft.)	Pipe Class	Bedding Class
Horizontal	1-6*	HE ∐*	C
	7-10	HE Ⅲ	C
	11-16	HE Ⅳ	C
	17+	Special Design	Modified
Vertical	1-6*	VE ∏*	C
	7-10	VE Ⅲ	C
	11-16	VE Ⅳ	C
	17+	Special Design	Modified

Pipe Class HE II D-Load=1000 Lbs/Ft/Ft(.01" Crack)
And VE II D-Load=1500 Lbs/Ft/Ft(Ultimate)

Pipe Class HE III D-Load = 1350 Lbs/Ft/Ft (.OI" Crack)
And VE III D-Load = 2000 Lbs/Ft/Ft (Ultimate)

Pipe Class HE IV D-Load = 2000 Lbs/Ft/Ft (.01" Crack)
And VE IV D-Load = 3000 Lbs/Ft/Ft (Ultimate)

*Note: HE III and VE III pipe required for depths of cover less than 2' for 15", 18" and 24" equivalent.

PIPE DIMENSIONS

POLYETHYLENE PIPE							
DIAMETER	HEIGHT OF MAXIMUM FILL (Ft)						
12" - 48"	16'						

POLY	VINYL CHLORIDE PIPE
DIAMETER	HEIGHT OF MAXIMUM FILL (Ft)
12" -48"	<i>1</i> 6'

MAXIMUM COVER FOR PLASTIC PIPE

MAXIMUM COVER HEIGHTS

Note: Height of fill (maximum cover) is measured from top of finished grade to outside top of pipe.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

COVER HEIGHT

	Names	Dates	Approve	· 1 /1 /1/	1 8
Designed By	EGR	09/85		// کا کہ State Drainage E	(chemore)
Drawn By	HSD	09/85	Revision	Sheet No.	Index No.
Checked By	EGR	09/85	00	2 of 5	205

^{*} For Informational Purposes Only Do Not Specify Wall Thickness Option B Wall Is Industry Standard

	ROUND PIPE - $2\frac{2}{3}'' \times \frac{1}{2}''' CORRUGATION$											
		Махі	imum He	ight Of F	iii (Ft. ,)						
			Sheet 1	hickness (Gage)	In Inches		Min.					
l D	Area	0.064	0.079	0.109	0.138	0./68	Cover					
(In)	(Sq. Ft.)	(16)	(14)	(12)	(10)	(8)	(Ft.)					
12	.79	100+	100+	NA	NA	NA						
15	1.23	100+	100+	NA	NA	NA						
18	1.77	100+	100+	100+	NA	NA						
21	2.40	100+	100+	100+	NA	NA	See					
24	3.14	100+	100+	100+	NA	NA	Sheet					
30	4.9/	85	100+	100+	NA	NA	1 of 5					
36	7./	71+	88	100+	100+	NA						
42	9.6	60+	76	100+	100+	NA						
48	12.6	53	66	93	100+	100+*						
54	16.0	NS	59	82	100+	100+*						
60	19.6	NS	NS	74	95	100+*						
66	23.8	NS	NS	NS	87	100+*						
72	28.3	NS	NS	NS	79	97*						
78	33.2	NS	NS	NS	NS	90*						
84	38.5	NS	NS	NS	NS	83*						

		Ma	ximum H	eight Of	Fill (Ft.	}	
				hickness (Gage)		•	Min.
D	Area	0.064	0.079	0.109	0./38	0.168	Cover
(In)	(Sq. Ft.)	(16)	(14)	(12)	(10)	(8)	(Ft.)
36	7./	81	100+	100+	NA	NA	
42	9.6	70	87	100+	NA	NA	
48	12.6	61	76	100+	100+	NA	
54	16.0	54	68	95	100+	NA	See
60	19.6	48	6/	85	100+	NA	Sheet Lof 5
66	23.8	44	55	78	100	100+*	1013
72	28.3	40	5/	71	91	100+*	
78	33.2	37	47	66	84	100+*	
84	38.5	35	43	6/	78	100+*	
90	44.2	32	40	57	73	90*	
96	50.3	NS	38	53	68	84*	
102	56.7	NS	36	50	64	79*	1
108	63.6	NS	NS	47	61	75*	
114	70.9	NS	NS	45	58	7/*	
120	78.5	NS	NS	42	55	67*	
132	95.0	NS	NS	NS	50	6/*	1

	ROUND	PIPE	- 5" x	I" CO	RRUGAT	rion 3)
		Мах	cimum He	eight Of .	Fill (Ft.	J	
			Sheet T	hickness ((Gage)	In Inches		Min.
D	Area	0.064	0.079	0.109	0./38	0.168	Cover
(In)	(Sq. Ft.)	(16)	(14)	(12)	(10)	(8)	(Ft.)
36	7./	72	90	100+	NA	NA	
42	9.6	62	77	100+	NA	NA	1
48	12.6	54	68	95	100+	NA	
54	16.0	48	60	84	100+	NA	See
60	19.6	43	54	76	98	NA	Sheet
66	23.8	39	49	69	89	100+*	lof 5
72	28.3	36	45	63	81	100*	1
78	33.2	33	41	58	75	92*	
84	38.5	3/	38	54	70	85*	1
90	44.2	29	36	50	65	80*	1
96	50.3	NS	34	47	61	75*	1
102	56.7	NS	32	44	57	70*	
108	63.6	NS	NS	42	54	66*	
114	70.9	NS	NS	40	5/	63*	
120	78.5	NS	NS	38	49	60*	
132	95.0	NS	NS	NS	44	54*	

	ARCH: SPIRAL							
PIPE	ARCH: SPIRAL	RIB:	<u>3</u> "	X	/" x	ΙΙ <u>Ι</u> '	RIB	SPACING
	PIPE ARCH	- 2 ද "	x ½"	C	ORF	RUGA	TION	

Span	Rise	Equiv. Round Pipe	Area	Minimum Sheet Thickness Required	Of Fill	Corner sure	Min. Cover
(In)	(In)	(In)		(In) (Ga)	4000	6000	(Ft.)
17	13	<i>1</i> 5	1.1	.064 (16)	12	14	
21	15	18	1.6	.064 (16)	10	14	
24	18	21	2.2	.064 (16)	7	13	
28	20	24	2.9	.064 (16)	5	//	
35	24	30	4.5	.064 (16)	NS	7	See
42	29	36	6.5	.064 (16)	NS	7	Sheet
49	33	42	8.9	.079 (14)	NS	6	1 of 5
57	38	48	11.6	.109 (12)	NS	8	
64	43	54	14.7	.109 (12)	NS	9	1
71	47	60	18.1	.138 (10)4	NS	10 ④	
77	52	66	21.9	.168 (8)* ④	5	10 ④	
83	57	72	26.0	.168 (8)* ④	5	10 4	

PIP	PIPE ARCH-3" x I" (123) and 5" x I" (23) CORR.											
		Maximum Height Of Fill(Ft.)										
		Equiv. Round		Minimum Sheet Thickness	Maximum Press	Corner sure	Min.					
Span	Rise	Pipe	Area	Required	Lbs/S	Sq.Ft.	Cover					
(In)	(In)	(In)	(Sq. Ft.)	(In)(Ga)	4000	6000	(Ft.)					
40	31	36	7.0	.079 (14)	8	12						
46	36	42	9.4	.079 (14)	8	13						
53	41	48	12.3	.079 (14)	8	13						
60	46	54	15.6	.079 (14)	8	13						
66	5/	60	19.3	.079 (14)	9	13						
73	55	66	23.2	.079 (14)	//	16						
81	59	72	27.4	.079 (14)	//	17	See					
87	63	78	32.1	.079 (14)	10	16	Sheet					
95	67	84	37.0	.079 (14)	//	17	1 of 5					
103	71	90	42.4	.109 (12)	10	15						
112	75	96	48.0	.109 (12)	10	16]					
117	79	102	54.2	.109 (12)	10	15]					
128	83	108	60.5	.138 (10)	9	14						
/37	87	114	67.4	.138 (10)	8	13						
142	91	120	74.5	.168 (8)	7	12						

	ROUND PIPE - SPIRAL RIB	
RIB	SPACING $(\frac{3}{4}^{"} \times \frac{3}{4}^{"} \times 7\frac{1}{2}^{"})$ or $(\frac{3}{4}^{"} \times 1^{"} \times 1/\frac{1}{2}^{"})$	

			Maximum	Height	Of Fill(Ft.)					
			Sheet Thickness In Inches (Gage)								
D (In.)	Area (Sq. Ft.)	0.064 (16)	0.079 (14)	0.109 (12)	0.138 (10)	0.168 (8)	Cover				
12	0.79	NA	NA	NA	NA	NA					
15	1.23	NA	NA	NA	NA	NA					
18	1.77	68	72	NA	NA	NA					
21	2.40	58	62	100+	NA	NA					
24	3.14	51	72	100+	NA	NA					
30	4.91	41	58	97	NA	NA					
36	7.1	34	48	81	NA	NA	See				
42	9.6	29	41	69	NA	NA	Sheet				
48	12.6	26	36	61	NA	NA	1 of 5				
54	16.0	23	32	54	NA	NA					
60	19.6	NS	29	49	NA	NA					
66	23.8	NS	26	44	NA	NA					
72	28.3	NS	24	40	NA	NA					
78	33.2	NS	NS	37	NA	NA					
84	38.5	NS	NS	35	NA	NA					
90	44.2	NS	NS	32	NA	NA					
96	50.3	NS	NS	30	NA	NA					
102	56.7	NS	NS	29	NA	NA					
108	63.6	NS	NS	27 ®	NA	NA					

 \triangle = $\frac{3}{4}$ " x /" x // $\frac{1}{2}$ " Only.

Notes:

Increase the minimum cover values shown on Sheet I of 5 by 6" for gage and size combinations below the heavy lines.

Height of fill (maximum cover) is measured from top of finished grade to outside top of pipe.

*Recorrugated end not available. May be considered for cross drain and side drain applications only.

NA - Not Available

NS - Not Suitable (For Highway H-20 Loadings)

- (1) Limited availability of this product. Check availability before specifying (generally limited to 3" x 1" corrugation pipe arch fabricated from 60" and smaller diameter round pipe in 12 ga. and thicker material).
- 2 360° perforated pipe arch (french drain pipe) is not recommended. Do not specify without checking suitability and availability.
- 3 5" x I" corrugated pipe is currently not manufactured for the Florida market. Check availability before specifying.
- 4) .109 in. (12 gage) for spiral rib, 8' maximum cover, $\frac{3}{4}$ " x I" x II $\frac{1}{2}$ " rib spacing (2 rib) only.

MAXIMUM COVER FOR CORRUGATED STEEL PIPE ROUND AND PIPE ARCH

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

COVER HEIGHT

	Names	Dates	Approve	d By	10
Designed By	EGR	09/85		// کا لکر State Drainage E	(chemose)
Drawn By	HSD	09/85	Revision	Sheet No.	Index No.
Checked By	EGR	09/85	00	3 of 5	205

	ROU	ND PIPL	-2 2 "	x ½" C	ORRUGA	TION					
		Maximum Height Of Fill(Ft.)									
		Si	Sheet Thickness In Inches (Gage)								
D (In)	Area (Sq. Ft.)	0.060 (16)	0.075 (14)	0.105 (12)	0.135 (10)	0.164 (8)	(Ft.)				
12	0.8	90	100+	NA	NA	NA					
<i>1</i> 5	1.2	72	90	NA	NA	NA	1				
18	1.8	59	75	100+	NA	NA	1				
21	2.4	52	65	92	NA	NA	1				
24	3./	44	56	79	NA	NA					
30	4.9	35 DR	44	63	NA	NA	See Sheet				
36	7./	NS	36 DR	52	68	NA	lof 5				
42	9.6	NS	NS	44 DR	58	NA]'"				
48	12.6	NS	NS	38 DR	50 DR	61	1				
54	15.9	NS	NS	34 DR	45 DR	54 DR	1				
60	19.6	NS	NS	NS	39 DR	49 DR	1				
66	23.8	NS	NS	NS	NS	44 DR	1				
72	28.3	NS	NS	NS	NS	40 DR	1				

	ROUND PIPE - 3" x I" CORRUGATION												
		Ма	ıximum H	eight Of	Fill (Ft.)								
		S	Sheet Thickness In Inches (Gage)										
D (In)	Area (Sg. Ft.)	0.060 (16)											
36	7./	33	42	60	NA	NA							
42	9.6	28	36	51	NA	NA							
48	12.6	24	3/	<i>4</i> 5	58	NA]						
54	15.9	21	28	39	5/	NA	1						
60	19.6	19	24	35	46	NA							
66	23.8	15 DR	22	32	42	51	See						
72	28.3	NS	20 DR	29	38	47	Sheet						
78	33.2	NS	15 DR	27	35	43	1 of 5						
84	38.5	NS	NS	24 DR	32	40							
90	44.2	NS	NS	23 DR	30	37	1						
96	50.3	NS	NS	21 DR	28 DR	34	1						
102	56.7	NS	NS	NS	26 DR	32	1						
108	63.6	NS	NS	NS	24 DR	30 DR]						
114	70.9	NS	NS	NS	NS	28 DR							
120	78.5	NS	NS	NS	NS	27 DR]						

		ROUN RIB	ID PIPE SPACIN	E - SF G (¾x	PIRAL RIE 3" x 7 ½")	3			
			Maximum Height Of Fill(Ft.)						
			Sheet	Thickness (Gage .	In Inches		Min. Cover		
D (In.)	Area (Sq. Ft.)	0.060 (16)			0.164 (8)	(Ft.)			
12	0.79	NA	NA	NA	NA	NA			
15	1.23	63 (1)	87()	NA	NA	NA			
18	1.77	55	76	NA	NA	NA			
21	2.40	47	65	NA	NA	NA			
24	3.14	41	57	NA	NA	NA			
30	4.91	33 DR	45	73	NA	NA			
36	7./	//27 4)	38 DR	61	NA	NA	See		
42	9.6	NS	//32 ④	52	NA	NA	Sheet		
48	12.6	NS	NS	46	65	NA	1 of 5		
54	16.0	NS	NS	40 DR	57	NA			
60	19.6	NS	NS	//36,4/	52	NA			
66	23.8	NS	NS	NS	47 DR	NA			
72	28.3	NS	NS	NS	/43 4	NA			
78	33.2	NS	NS	NS	39 4	NA			
84	<i>38</i> .5	NS	NS	NS	34 4	NA			
90	44.2	NS	NS	NS	30(1)(3)(4)	NA			
96	50.3	NS	NS	NS	/27(D3(4)	NA			

Special installation required.
Refer to AASHTO Standard Specifications for Highway Bridges or ASTM B788-88 and manufacturer's recommendations.

P	PIPE ARCH - 23 x 2 CORRUGATION @											
						m Height V(Ft.)						
Span	Span Rise I		Area	Minimum Sheet Thickness Required		Corner Lbs/Sq.Ft.	Min. Cover					
(In)	(In)	Pipe (In)		(In) (Ga)	4000	6000	(Ft.)					
17	13	<i>1</i> 5	1.1	.060 (16)	12	15						
21	<i>1</i> 5	18	1.6	.060 (16)	10	14						
24	18	21	2.2	.060 (16)	7	/3						
28	20	24	2.9	.075 (14)	5	11						
35	24	30	4.5	.075 (14)	NS	7						
42	29	36	6.5	.105 (12)	NS	7	See Sheet					
49	33	42	8.9	.105 (12)	NS	6	lof 5					
57	38	48	// . 6	.135 (10)	NS	8						
64	43	5 4	14.7	./35 (10)	NS	9						
71	47	60	18.1	.164 (8)	NS	10						
77	52	66	21.9	.164 (8)	NS	10						
83	57	72	26.0	.164 (8)	NS	10						

	PIP	E ARC	:H - 3'	" x I" CO	RRUGATIO	ON (1) (2)	
				44*	Maximum Of Fill		
Span	Equiv. Round n Rise Pipe		Area	Minimum Sheet Thickness Required	l	Corner Lbs/Sq.Ft.	Min. Cover
(In)	(In)	(In)	(Sq. Ft.)	(In) (Ga)	4000	6000	(Ft.)
40	3/	36	7.0	.060 (16)	8	12	
46	36	42	9.4	.060 (16)	8	13	
53	41	48	12.3	.060 (16)	8	13	
60	46	54	<i>15.</i> 6	.075 (14)	8	13	See
66	5/	60	19.3	.075 (14)	8	13	Sheet
73	55	66	23.2	.105 (12)	//	16	1 of 5
81	59	72	27.4	.105 (12)	11	17	
87	63	78	32.1	.105 (12)	10	16	
95	67	84	37.0	.105 (12)	//	17	
103	71	90	42.4	.135 (10)	10	15	
112	75	96	48.0	./35 (10)	10	16	
117	79	102	5 4. 2	.164 (8)	10	15	

	PIPE ARCH - SPIRAL RIB RIB SPACING (3 x 3 x 7 ½ ")													
Minimum Of Fill (Ft.) Sheet														
Span	Equiv. Round From Rise Pipe Area Sheet Thickness Required Maximum Corner Pressure - Lbs/Sq.Ft.													
(In)	(In)	(In)	(Sq. Ft.)	4000	6000	(Ft.)								
16	14	<i>1</i> 5	1.2	.060 (16)	12	13								
20	16	18	1.7	.060 (16)	10	12								
23	19	21	2.3	.060 (16)	7	//								
27	21	24	3.0	.060 (16)	5	10								
33	26	30	4.7	.075 (14)	NS	9								
40	3/	36	7.0	.075 (14)	NS	8	See Sheet							
46	36	42	9.4	.105 (12)	NS	8	1 of 5							
53	41	48	12.3	.105 (12)	NS	8								
60	46	54	15.6	.105 (10)	NS	8								
66	5/	60	19.3	./35 (10)	NS	8								
73	55	66	23.2	/.1 <i>35/(10)</i> 4 /	NS	8								
81	59	72	27.4	/.135/(10)4/	NS	8								

MAXIMUM COVER FOR CORRUGATED ALUMINUM ALLOY ROUND PIPE AND PIPE ARCH

Notes

Increase the minimum cover values shown on Sheet I of 5 by 6" for gage and size combinations below the heavy lines.

Height of fill (maximum cover) is measured from top of finished grade to outside top of pipe.

- NA Not Available
- NS Not Suitable (For Highway H-20 Loadings)
- DR Design Review is recommended for each specific application. The review should identify any special handling, installation, backfill procedures, and construction load restrictions which may be required. (The review performed by the designer does not relieve the contractor from analyzing and taking any necessasry precautions required to protect partially or completely constructed pipe from the equipment used during construction.)

 (NOTE: The DESIGNER may use a thicker gage in lieu of the Design Review
- () Limited availability of this product. Check availability before specifying.
- ② 360° perforated pipe (french drain pipe) is not recommended in the pipe arch shape. Do not specify without checking both for suitability and availability.
- 3 This size and gage combination must be strutted during installation per manufacturers recommendations. Extra care will be required during handling and installation.
- (NOTE: The DESIGNER may use a thicker gage in lieu of the Design Review.) ④ Use of this size and gage combination must be approved by the State Drainage Engineer.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

COVER HEIGHT

	Names	Dates	Approve		1 0
Designed By	EGR	09/85		State Drainage E	chemore Engineer
Drawn By	HSD	09/85	Revision	Sheet No.	Index No.
Checked By	EGR	09/85	00	4 of 5	205

С	ombination	Metal Thic	kness, Rei	nforcing Ri	b Type, an	d Rib Spac	ing
		Round	Shape - H	<u>5 20 Live</u> imum Heigh	<u>Load</u> nt of Cover	(Ft.)	
Diameter (FtIn.)	Area (Sq. Ft.)	1.00 Ft.	1.50 Ft.	2.00 Ft.	2.50 Ft.	3.00 Ft.	3.50 Ft.
5-0	19	.125 (45)	.100	.100 (31)	.100 (31)	.100 (31)	.100 (31)
5-6	23	.125 -II - 18	.100	.100	.100	.100	.100
6-0	28	(37)	(25)	(25)	(25)	(25)	(25)
6-6	32	.125 -II - 18	.100	.100	.100	.100	.100
7-0	38	(32)	(22)	(22)	(22)	(22)	(22)
7-6	44	.125 -II - 9	.150	.100	.100	.100	.100
8-0	50	(28)	(37)	(19)	(19)	(19)	(19)
8-6	56	.125 - IV - 9	.125 -∏-18	.100	.100	.100	.100
9-0	63	(25)	(25)	(17)	(17)	(17)	(17)
9-6	71	.125 - IV - 9	.125 -II - 18	.125	.100	.100	.100
10-0	79	(22)	(22)	(22)	(15)	(15)	(15)
10 -6	87	.175- <u>IV</u> -9	.125 - II - 18	.125 -II - 27	.100	.100	.100
11 -0	95	(32)	(20)	(20)	(14)	(14)	(14)
II -6	104		.125 - IV - 18	.125 -II - 27	.125	.100	.100
I2 -0	114		(18)	(18)	(18)	(12)	(12)
12-6	124		./50 - IV - /8	.125 -II - 27	.150	.125	,125
13-0	134		(23)	(17)	(23)	(17)	(17)
13 -6	145		.125 -IV -9	.125 - IV - 27	.125 -II - 27	.150	.150
14 -0	156		(16)	(16)	(16)	(21)	(21)
14 - 6	167		.125 -II-54	.125 -I∇ -9	.125 -IV - 27	.125 -II - 27	.125 -∏-54
15 - 0	179		(15)	(15)	(15)	(15)	(15)
15 - 6	191		./50 - IV - 9	.125 - IV - 18	.125 - II - 27	.150 -II - 54	3.8/II/I350
16 - 0	204		(/8)	(14)	(14)	(18)	(18)
16 - 6	217		.225 -IV -9	.150 -1⊻ -18	.150 -∏-27	.150 -II - 27	.150 -II-27
17 - 0	231		(27)	(17)	(17)	(17)	(17)
17 - 6 18 - 0	245 259			.175-IV-18 (19)	.175-II-27 (19)	.175-II-27 (19)	.175-II-27 (19)
18 - 6 19 - 0	274 289			.175- <u>IV</u> -9 (18)	.175- <u>IV</u> -27 (18)	.175-II-27 (18)	.175-II-27 (18)
19-6	305			.200 - <u>IV</u> - 9 (20)	.200 - IX - 27 (20)	.200-II-27 (20)	.200 -II - 27 (20)

Aluminum Structural Plate

Height of Cover Limits*

٦						Structura			
1						of Cover Li			
1		Ca	ombinatio					and Rib Spac	ing
4	1					- HS 20 L		(51)	
4	<u> </u>				Minimum Height of Cover (Ft.)				
	Span (FtIn.)	Rise (FtIn.)	Area (Sq.Ft.)	1.00	1.50	2.00	2.50	3.00	3.50
٦	5-0	1-9	7	.125	.100	.100	.100	./00	.100
1		2-3	9	(45)	(3/)	(31)	(31)	(31)	(31)
┨		2-7	10						
1	6-0	1-10	8	.125 -II-18	.100	./00	.100	.100	./00
4		2-4	10	(37)	(25)	(25)	(25)	(25)	(25)
1		2-9 3-2	13 15						
╛	7-0	2-4	12	.125 -II-18	.100	,100	./00	.100	./00
٦	'"	2-10	15	(32)	(22)	(22)	(22)	(22)	(22)
1		3-3	18	, 52,					
┪		3-8	20						
1	8-0	2-11	17	.125 -II-9	.150 (37)	.100 (19)	.100 (19)	.100 (19)	.100 (19)
┨		3-4 4-2	20 26	(28)	13/)	1757	(79)	(19)	(197
1		4-2	26						
4	9-0	2-11	19	.125 -1▼-9	.125 -II-18	.100	./00	.100	.100
1		3-10	26	(25)	(25)	(17)	(17)	(17)	(17)
╛		4-8	33						
1	10-0	3-6	25	.125 -IV -9	.125 -II-18	./25	./00	.100	.100
1		4-5	33	(22)	(22)	(22)	(15)	(15)	(15)
7		5-2	41						
1	11-0			./75-IX-9	.125-II-18	./25 -II - 27	.100	.100	./00
┨	<i>"-0</i>	3-6 4-6	28 37	(32)	(20)	(20)	(14)	(14)	(14)
1		5-8	50						
Н	—						105	100	100
1	12-0	4-1 5-0	35 45		,125 -IV -18 (18)	.125 -II - 27 (18)	.125 (18)	.100 (12)	.100 (12)
4		6-3	59		(10)	(707	1,0,	,,,,,,	1,2,
۱۲									
╛	/3-0	4-1	38		./50 <i>-IX</i> -/8	.125 -II - 27	./50	.100	./00
1		5-1 5-11	49 59		(23)	(17)	(23)	()	(11)
1		6-9	70						
1	14-0	4-8	47		.125 -IV -9	.125 -IV - 27	.125 -II-27	.100	.100
		5-7 6-5	58		(16)	(16)	(16)	(11)	(11)
7		7-3	70 81						
1	15-0	4-8	50		.125 - IV - 9	.125 -IX - 27	.125 -II - 27	.125	.125
Н	""	5-8 6-7	63		(15)	(15)	(15)	(15)	(15)
7		7-5	75 87						
J		7-9	93						
	16-0	5-3	60		.150 -17 - 9	.125 -IV -18	.125 -II - 27	.150 (18)	.125 (14)
	1	6-2 7-1	73 86		(18)	(14)	(14)	(10)	(14)
		7-11	99						
		8-3	105						
	17-0	5-3	64		.225 -I∇ -9	./50 -IX - 18	.125 -II - 27	.175	.150
		6-3	78		(27)	(17)	(13)	(20)	(17)
		7-2	92						
		8-0	105						
	1	8-10	119	l	I	l	1		I

.175 -IV -18

(19)

(11)

.125-IV-27

(11)

(12)

.125-IV-9 .125-IV-27

(22)

(19)

.125 -IV - 54 .125 - IV - 54

	Aluminum Structural Plate	
	Height of Cover Limits*	
Combination	Metal Thickness, Reinforcing Rib Type, and Rib Spacing	
	Undernass Shape - HS 20 Live Load	

	Combin	1011011 1111		pass Shape		Live Load	a mo spac	,,,,d		000	70
				М	'inimum He	ight of Cov	er (Ft.)				
Span In.)	Rise (FtIn.)	Area (Sq.Ft.)	1.00	1.50	2.00	2.50	3.00	3.50	Span (FtIn.)	Rise (Ft-In)	Area (Sq-F
5-/	5-9	28	.125 - II - 18 (29)	.100 (25)	.100 (25)	.100 (25)	.100 (25)	.100 (25)	6-7 6-11	5-8 5-9	30 32
6-3 6-3 6-2 6-4 6-3 6-5	6-1 6-5 6-11 7-3 7-9 8-1	30 32 34 37 39 42	.125-II-18 (25)	.100 (22)	.100 (22)	./00 (22)	.100 (22)	./00 (22)	7-3 7-9 8-1 8-5 8-10	5-II 6-0 6-I 6-3 6-4	34 37 39 42 45
2-1	11-0	106		.125 -IV -18 (14)	.125 -II - 27 (14)	.125 (14)	.100 (12)	.100 (12)	9-3 9-7 9-11	6-5 6-6 6-8	47 50 53
2-10 13-0	11-2 12-0	114 124		.150 -1 <u>V</u> -18 (13)	.125-II-27 (13)	.150 (13)	.125 (13)	.125 (13)	10-3 10-9 11-1	6-9 6-10 7-0	56 58 6/
3-8 4-0	12-4 12-11	133 143		.125-IV-9 (12)	.125 -IV - 27 (12)	.125 - II - 27 (12)	.125 -II- 54 (12)	.125 -II-54 (12)	//-5 //-9	7-1 7-2	64 68
4-6 4-8	13-5 14-1	/55 /65		.125- I ⊻-9 (Ⅱ)	.125 -IV - 27 (11)	.125 - II - 27 (11)	.125 -II - 54 (II)	,125 -II-54 (II)	12-3 12-7 12-11	7-3 7-5 7-6	71 74 77
5-5 5-6	14-5 15-2	177 190		./50- <i>II</i> V-9 (//)	.125 -IV -18 (11)	.125 - II - 27 (11)	.125 -II - 27 (II)	.125 -II-2 (II)	13-1 13-1	8-2 8-4	83 87
6-2 6-6 6-8	/5-6 /6-0 /6-4	200 208 215		.225-IV-9 (10)	.150 - IX - 18 (10)	.150 - II - 27 (10)	.150 -II - 27 (10)	.150 -II-27 (10)	13-11 14-0 13-11	8-5 8-7 9-5	90 94 102
	•	-	•						14-3	9-7	106

Aluminum Structural Plate
Height of Cover Limits*
Combination Metal Thickness, Reinforcing Rib Type, and Rib Spacing
Pipe-Arch Shape- HS 20 Live Load

			Minimum Height of Cover (Ft.)						
Span (FtIn.)	Rise (Ft-In)	Area (Sq-Ft)	1.00	1.50	2.00	2.50	3.00	3.50	
6-7 6-11	5-8 5-9	30 32	.125 -II-18 (25)	.100 (22)	.100 { 22 }	.100 (22)	.100 (22)	.100 (22)	
7-3 7-9 8-1	5-11 6-0 6-1	34 37 39	.125 - IX - 18 (22)	./50 (22)	.100 (19)	.100 (19)	.100 (19)	.100 (19)	
8-5 8-10	6-3 6-4	42 45	.125 - IV - 9 (19)	.125 -II-18 (19)	.100 (17)	.100 (17)	.100 (17)	./00	
9-3 9-7 9-11	6-5 6-6 6-8	47 50 53	.125 -IX -9 (17)	.125 -II - 18 (17)	.125 (17)	.100 (15)	.100 (15)	.100 (15)	
10-3 10-9 11-1	6-9 6-10 7-0	56 58 6/	.175 - IV - 9 (16)	.125 - II - 18 (16)	.125 -II - 27 (16)	.100 (14)	.100 (14)	.100 (14)	
/I-5 /I-9	7-1 7-2	64 68		.125 - IX - 18 (14)	.125 -II - 27 (14)	.125 (14)	.100 (12)	.100 (12)	
12-3 12-7 12-11 13-1 13-1	7-3 7-5 7-6 8-2 8-4	71 74 77 83 87		.150 -1 <u>V</u> -18 (13)	.125-II-27 (13)	.150 (13)	.100 (11)	.100 ()	
13-11 14-0 13-11	8-5 8-7 9-5	90 94 102		.125 - IV - 9 (12)	.125 - IV - 27 (12)	.125 -II - 27 (12)	.100 (11)	.100 (11)	
14-3 14-8 14-11	9-7 9-8 9-10	106 110 114		.125 -IV -9 (11)	.125 - IV - 27 (11)	.125 -II - 27 ()	.125 (11)	.125 (11)	
15-4 15-7 16-1	10-0 10-2 10-4	119 123 128		.150-I∇-9 (Ⅱ)	.125-IV-18 (11)	.125 -II - 27 (11)	.125 -II - 54 (11)	.125 (11)	
16 - 4	10-6	132		.225 -IV -9 (10)	.150 -IV - 18 (10)	.125 -II - 27 (10)	.125 -II - 54 (10)	.125 -II-54 (10)	

ALUMINUM STRUCTURAL PLATE NOTES

- I. Allowable cover (minimum & maximum) is measured from the outside valley of crown plate to the bottom of flexible pavement or from the outside valley of the crown plate to the top of rigid pavement. Minimum cover must be maintained in unpaved areas. Maximum cover is measured at the highest fill and/or the highest pavement elevation.
- 2. To find the minimum material requirements for the aluminum structural plate structure:
 - a. Select the span in the left hand column that is equal to or larger than structure size required.
 - b. Select the cover in the top row that is equal to or smaller than that required for the
 - c. Intersect appropriate span and cover to find the appropriate plate.

Example: Round Pipe, Span = 17'-0", Height of Cover = 2'-7" (use 2.5 ft. in table).
Ans: .I50/II/27 (17)

The table selections show metal thickness, rib type, rib spacing and maximum cover.

Example: .150/II/27 = 0.150" thick plate structure with Type II rib at 27" on centers on the crown.

Number (17) in parenthesis below combination indicates maximum cover in feet for the given combination of plate thickness, rib type, and rib spacing.

- 3. Arch shapes shown are single radius and have a rise-to-span ratio of 0.30 to 0.53 Structures with rise-to-span ratios of less than 0.30 are typically not used because of structural considerations.
- 4. Tables based on HS 20 wheel loads.

MINIMUM AND MAXIMUM COVER FOR ALUMINUM STRUCTURAL PLATE

DESIGN NOTES

- I. The plans must call out size, metal thickness, reinforcing rib type and rib spacing.
- 2. Pipe-arch and underpass shapes will generate high corner bearing pressures against the sidefill and foundation. The height of cover is directly affected by these bearing pressures. The surrounding soil and foundation must be checked to ensure that they to react against these pressures to avoid inducing excessive strain in plate.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

COVER HEIGHT

	Names Dates		Approved By					
Designed By	WPH	7/97	State Drainage Engineer					
Drawn By	JDT	7/97	Revision	Sheet No.	Index No.			
Checked By	WPH	7/97	00	5 of 5	<i>20</i> 5			

* Number in () below combination indicates maximum cover for the given combination plate thickness, rib type and rib spacing

5-9 6-9 7-8 8-6

6-4 7-4 8-2 9-0 9-5

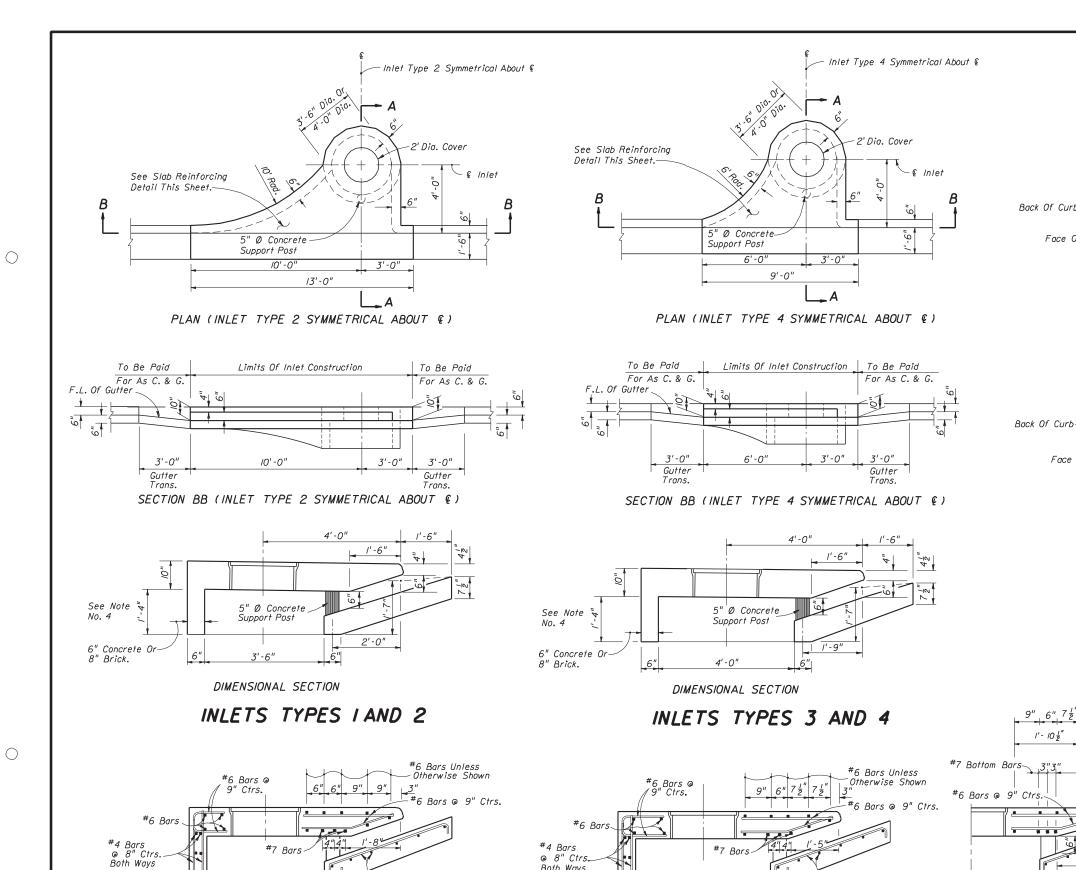
19-0

126

87

103 118 133

* All MAXIMUM COVER DEPTHS ARE GIVEN IN FEET. (See Note Number 2 Under Structural Plate Notes).



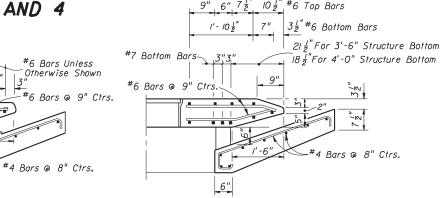
Both Ways

#4 Bars @ 8" Ctrs.

3'-6" Diameter

REINFORCING SECTION

3'-6" DIA. STRUCTURE BOTTOM (SECTION AA)



Face Of Curb

Face Of Curb

#6 Bars @ 9'

REINFORCING SECTION 4'-0" DIA. STRUCTURE BOTTOM (SECTION AA)

DIMENSION & REINFORCING HALF SECTION TYPES A & E CURB (HALF SECTION AA) (TYPE E GUTTER SHOWN)

TRANSVERSE SECTIONS FOR INLETS TYPES 1, 2, 3 & 4

4'-0" Diameter



INLETS TYPES 2 AND 4

SLAB REINFORCING

INLETS TYPES I AND 3

- GENERAL NOTES The finished grade and slope of the inlet tops are to conform with the finished cross slope and grade of the proposed sidewalk and/or border.
- 2. When inlets are to be constructed on a curve, refer to the plans to determine the radius and, where necessary, modify the inlet details accordingly. Bend steel when necessary.

5" Ø Concrete Support Post

5" Ø Concrete Support Post

#6 Bars

#6 Bars @ 9'

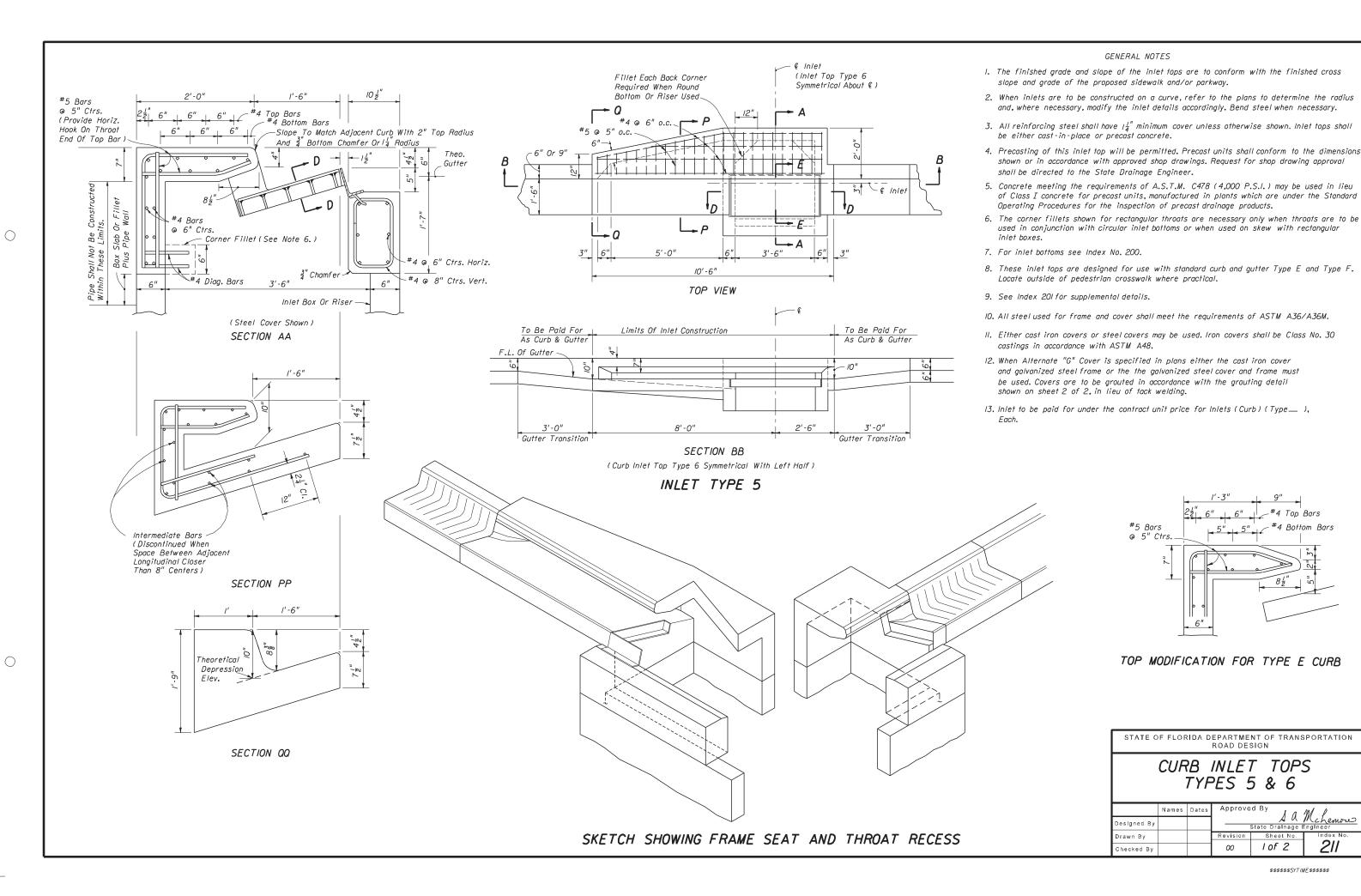
#6 Bars

- 3. All steel in inlet top shall have t_{i}^{μ} minimum cover unless otherwise shown. Inlet tops shall be either cast-in-place or precast concrete.
- 4. The rear wall portion of inlet tops Types 1, 2, 3 & 4 may be constructed with brick. Dowels to top slab required.
- 6. For supplemental details see Index No. 201.
- 5. Only round concrete support post will be acceptable.
- 7. These inlets are to be used with Curb and Gutter Types E and F. Locate outside of pedestrain crosswalk where practical.
- 8. For structure bottoms see Index No. 200.
- 9. Inlet to be paid for under the contract unit price for Inlets (Curb) (Type __), Each.

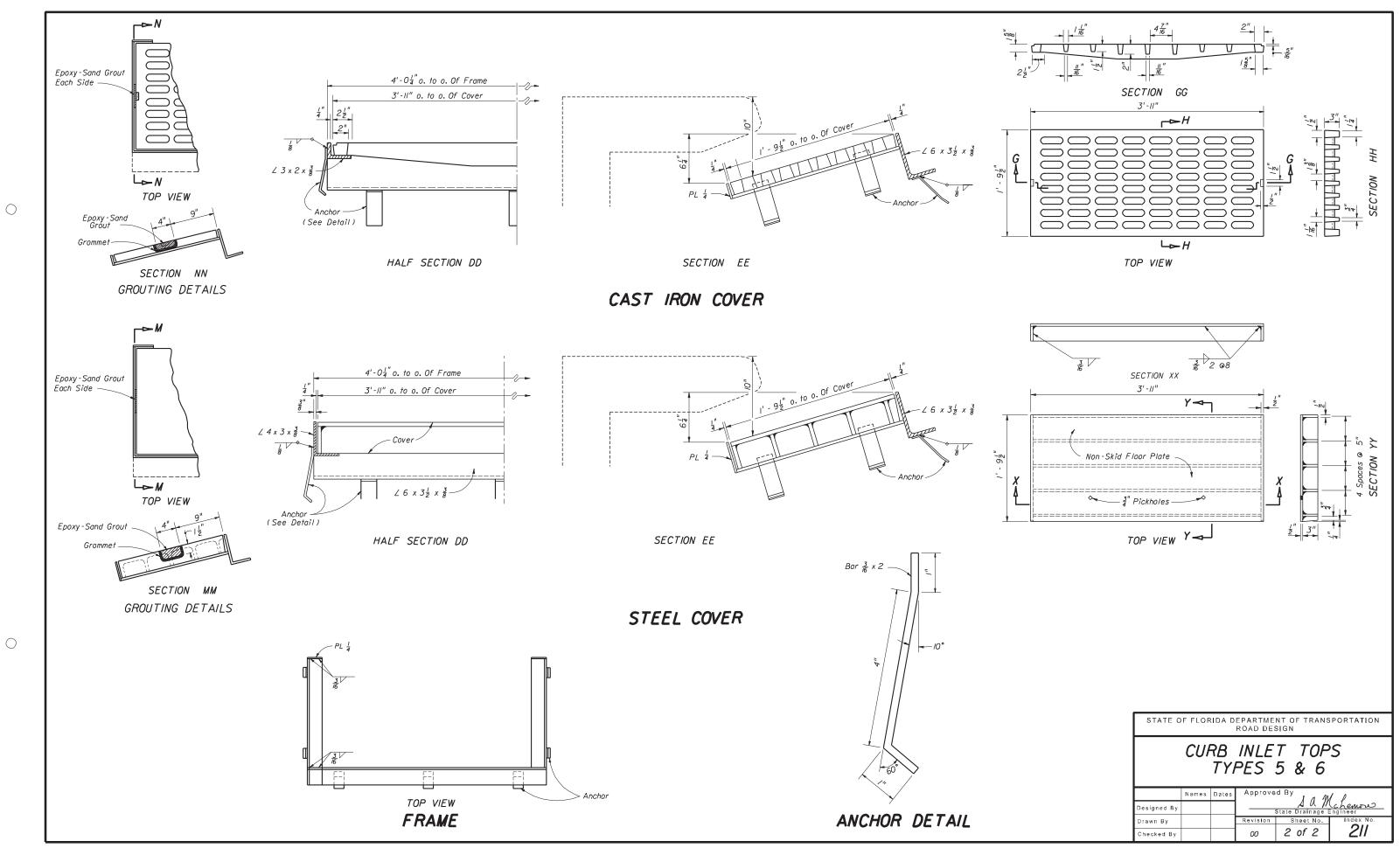
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

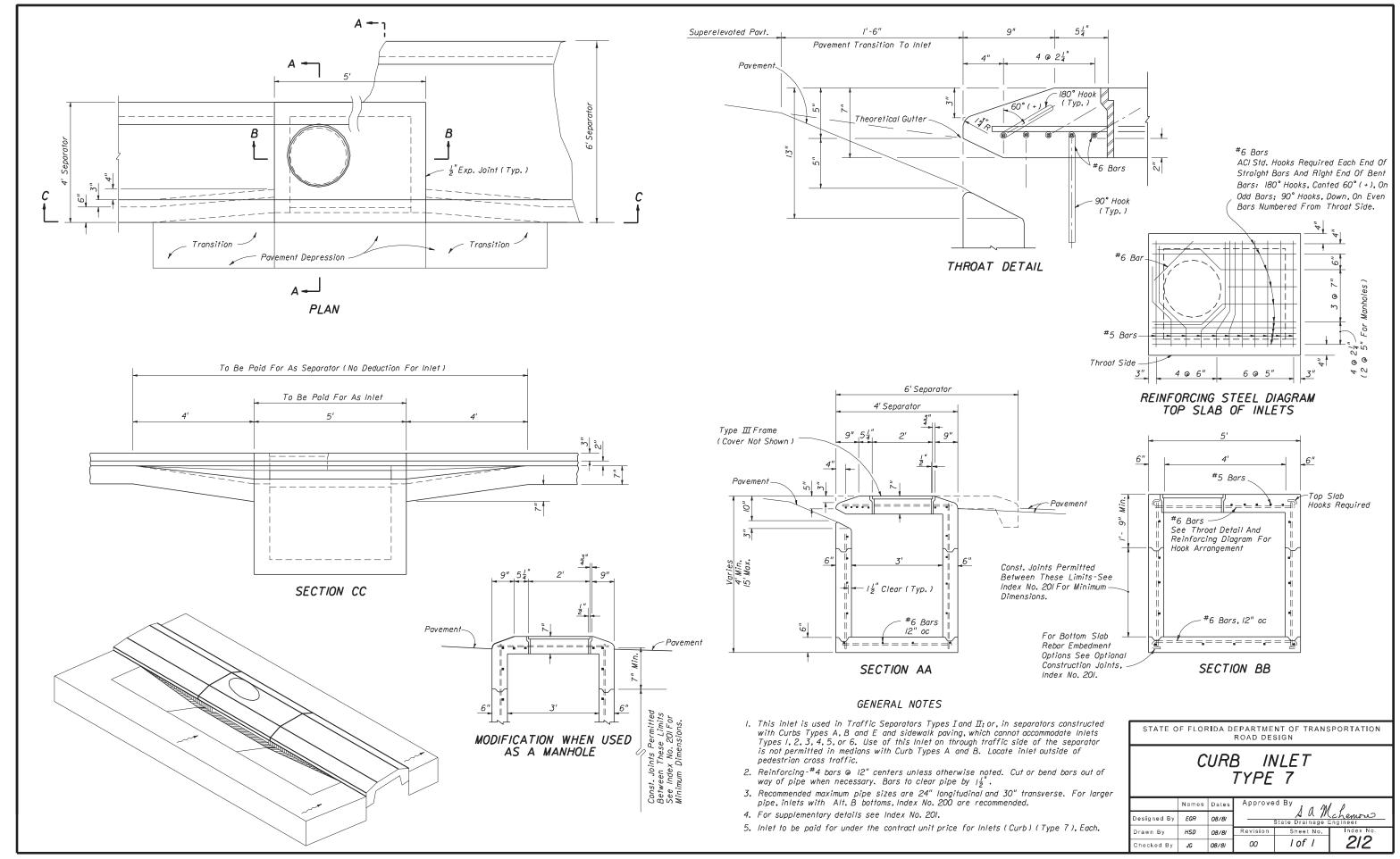
> CURB INLET TOPS TYPES 1.2.3.& 4

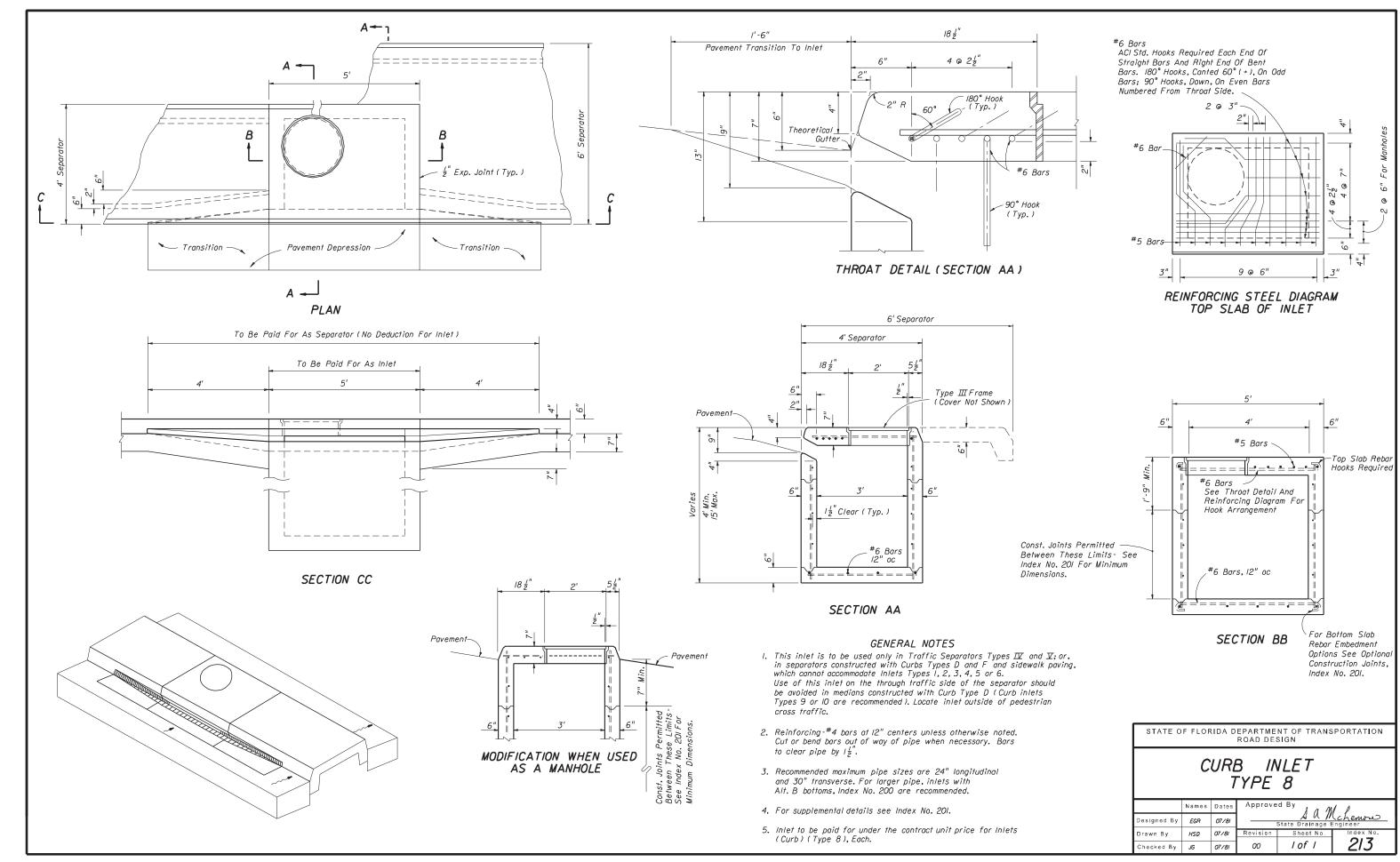
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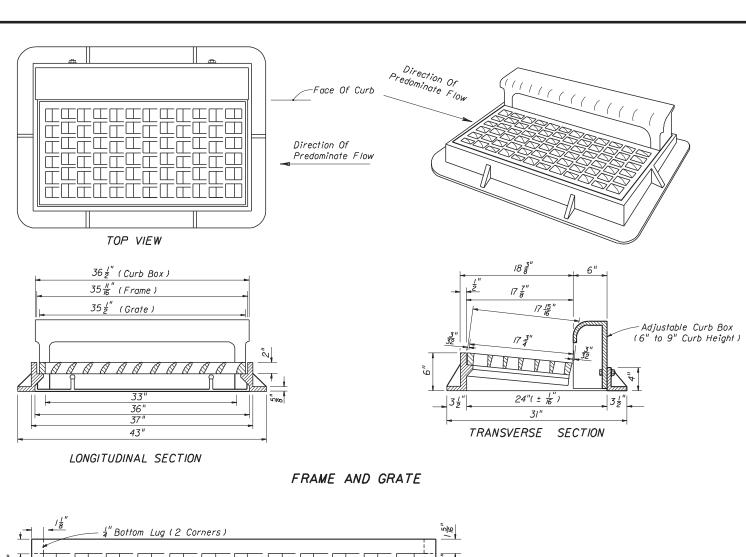


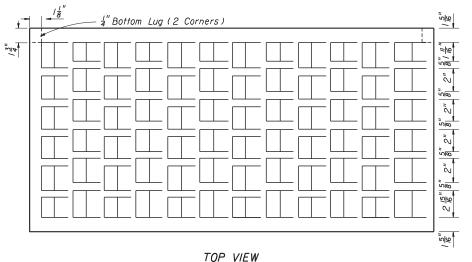
211

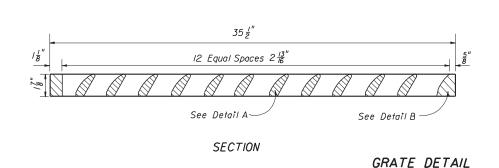


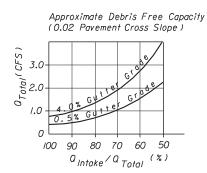




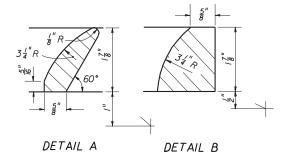


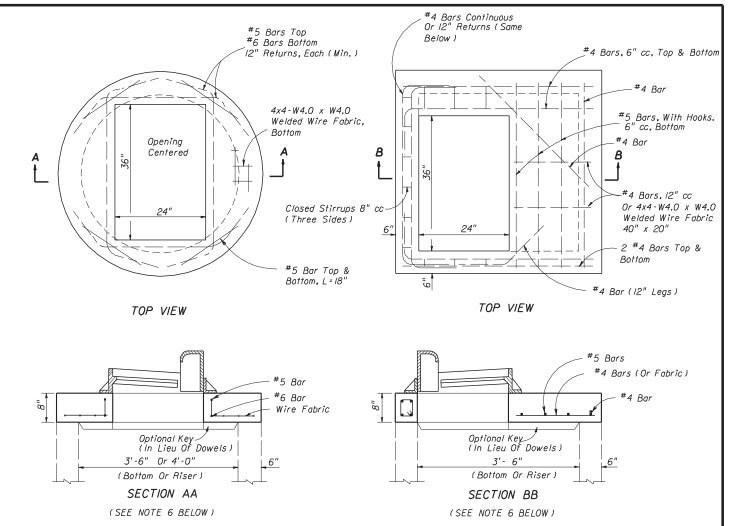






EFFICIENCY CURVE

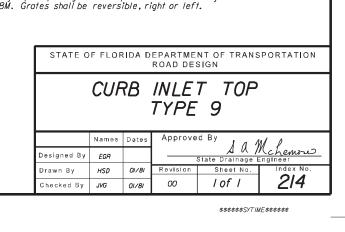


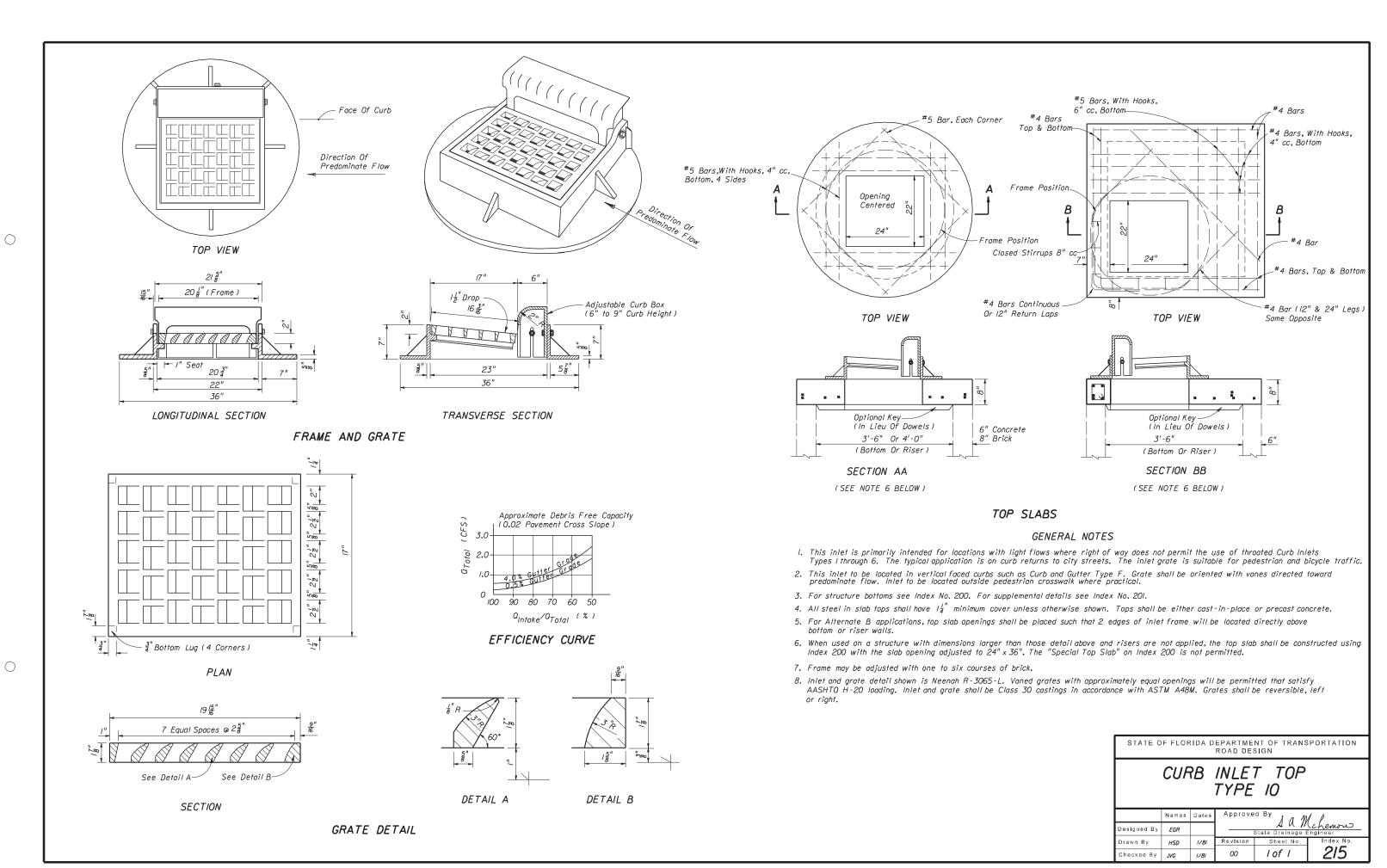


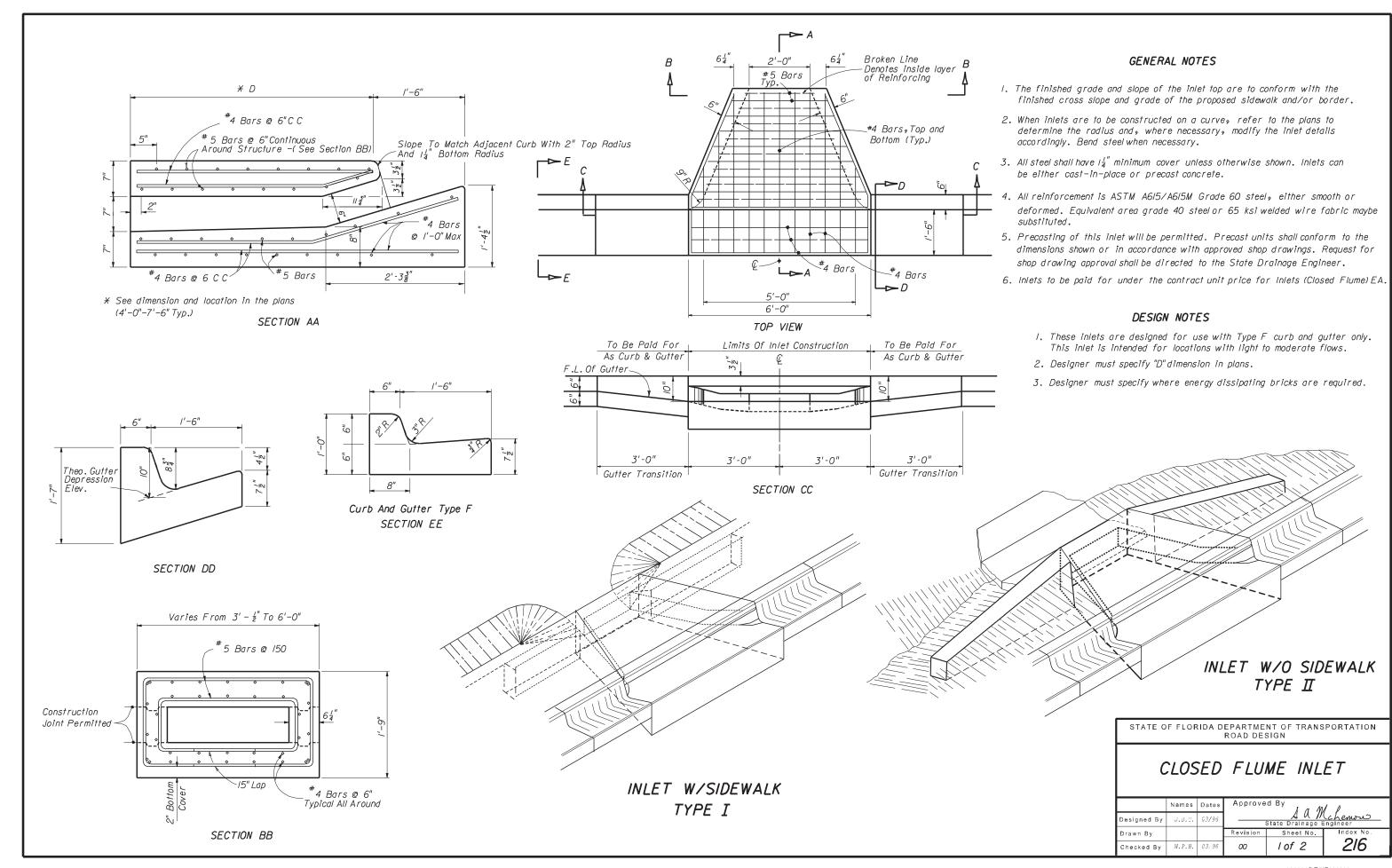
TOP SLABS

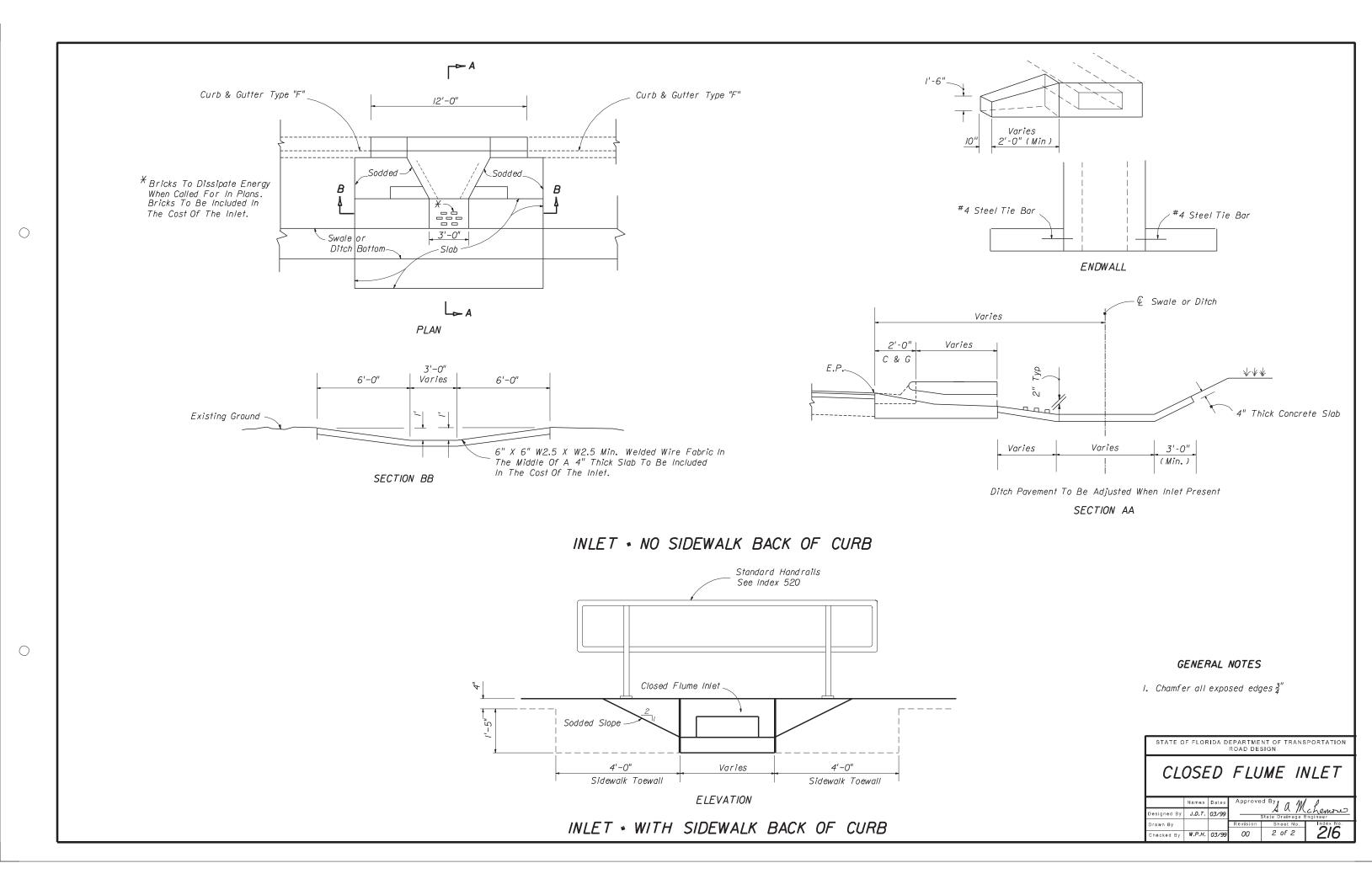
GENERAL NOTES

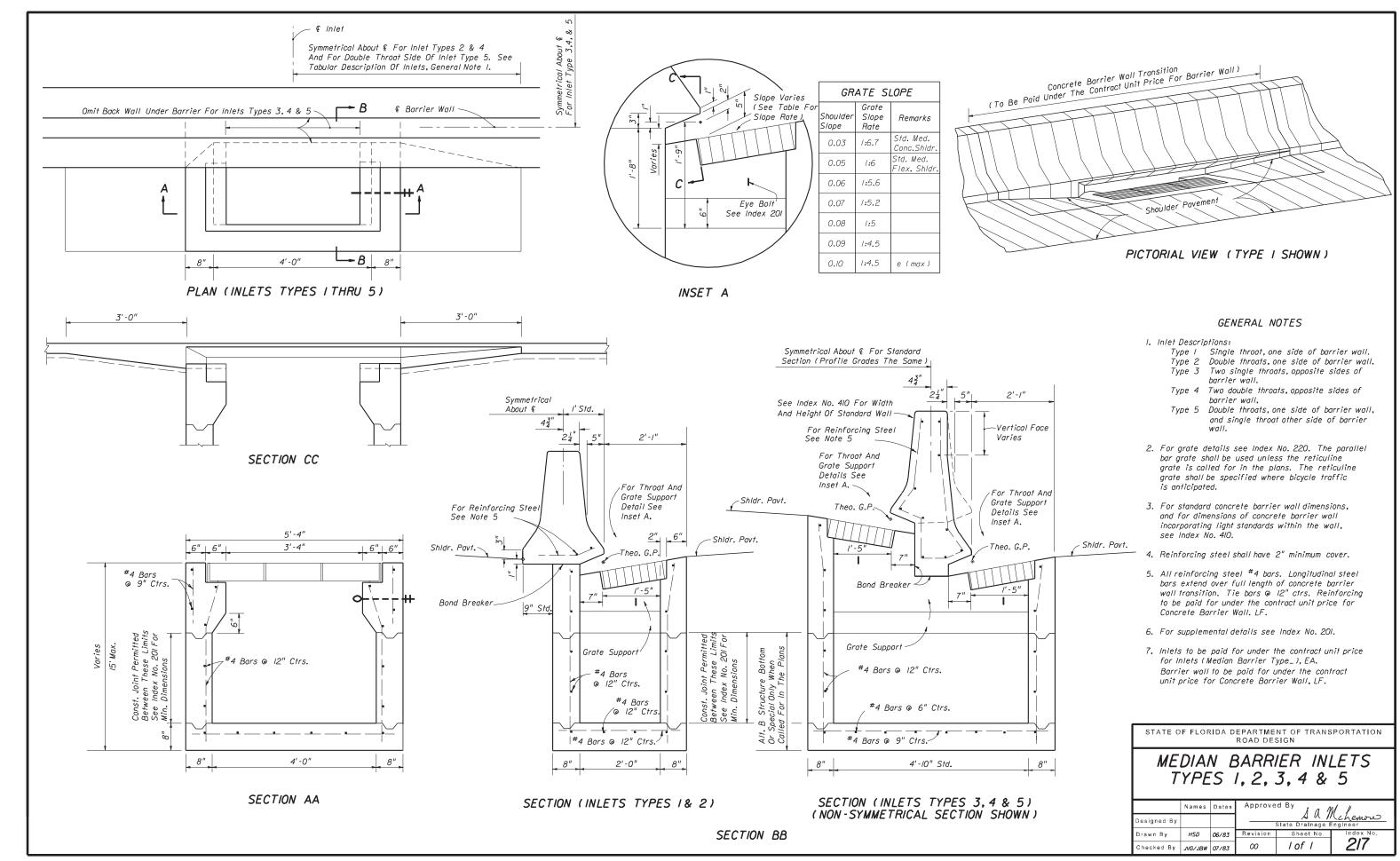
- I. This inlet is primarily intended for locations with light to moderate flows where right of way does not permit the use of throated Curb Inlets Types I through 6. The typical application is on curb returns to city streets. The inlet grate is suitable for pedestrian and bicycle traffic.
- 2. This inlet to be located in vertical faced curbs such as Curb and Gutter Type F. Grate shall be oriented with vanes directed toward predominate flow. Inlet to be located outside pedestrian crosswalk where practical.
- 3. For structure bottoms see Index No. 200. For supplemental details see Index No. 201.
- 4. All steel in slab tops shall have $l_4^{\#}$ minimum cover unless otherwise shown. Tops shall be either cast-in-place or precast concrete.
- 5. For Alternate B applications, top slab openings shall be placed such that 2 edges of inlet frame will be located directly above bottom wall or riser wall.
- 6. When used on a structure with dimensions larger than those detailed above and risers are not applied, the top slab shall be constructed using Index 200 with the slab opening adjusted to 24" x 36". The "Special Top Slab" on Index 200 is not permitted.
- 7. Frame may be adjusted with one to six courses of brick.
- 8. Inlet and grate detail shown is Neenah R-3067-L. Vaned grates with approximately equal openings will be permitted that satisfy AASHTO H-20 loading. Inlet and grate shall be Class 30 castings in accordance with ASTM A48M. Grates shall be reversible, right or left.

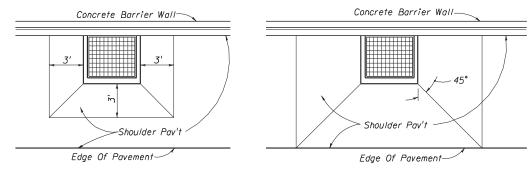








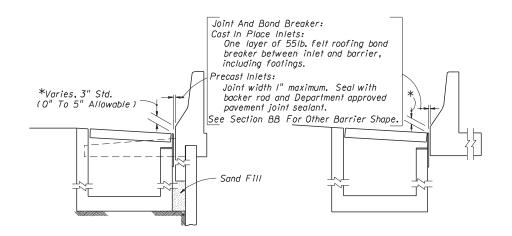




LOW SIDE SUPERELEVATION HIGH SIDE TRANSITION

PAVEMENT WARP FOR SHOULDERS IN SUPERELEVATION

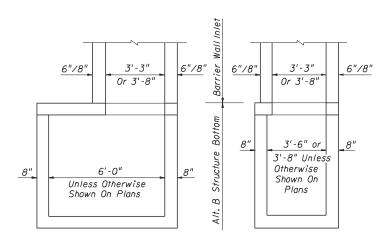
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SINGLE FACE ROADWAY BARRIER

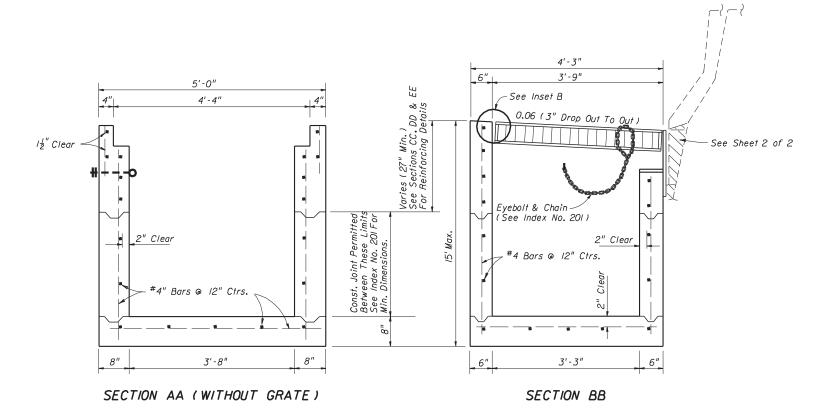
INLET SECTION AT WALLS

BARRIER WALL / RETAINING WALL



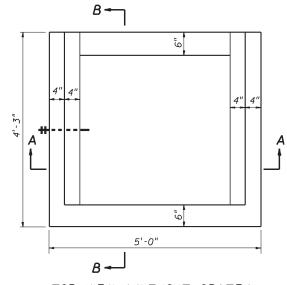
Note: Alt. B Structure Bottom Only. See Index No. 200.

INLET WITH STRUCTURE BOTTOM



GENERAL NOTES

- I. This inlet is primarily intended for use adjacent to concrete barrier walls on paved shoulders. Use of the inlet adjacent to other wall types shall be approved by the Drainage Engineer. The inlet is suitable for bicycle and occasional pedestrian traffic. It is not intended for use in curb and gutter or other areas where throated inlets are required, nor areas subject to high debris.
- 2. Inlets located in embankments constructed with earth anchored retaining wall shall be designed with minimum depths to reduce adverse impact on the anchorage system. Runs of pipe parallel to and near anchored wall shall be avoided wherever practical. Special coordination must be exercised during the design and construction of storm water systems within anchored wall systems.
- 3. Inlet bottoms and/or tops may be either precast or cast-in-place. Whether cast as a single unit or as multiple segments, and whether precast or cast-in-place, the upper 2'-3" of the inlet shall be reinforced in accordance with sections CC, DD and EE.
- 4. Exposed edges shall be chamfered $\frac{3}{4}$.
- 5. When Alternate G grate is specified in the plans, the grate is to be hot dipped galvanized after fabrication. Field installation of the filler bar called for in Inset B will not be permitted, thereby requiring tolerance adjustment during fabrication and/or casting, or, matching grate to structure prior to galvanizing.
- 6. For supplemental details see Index Nos. 200 and 201.
- 7. Inlets to be paid for under the contract unit for Inlets (Barrier Wall), Each.

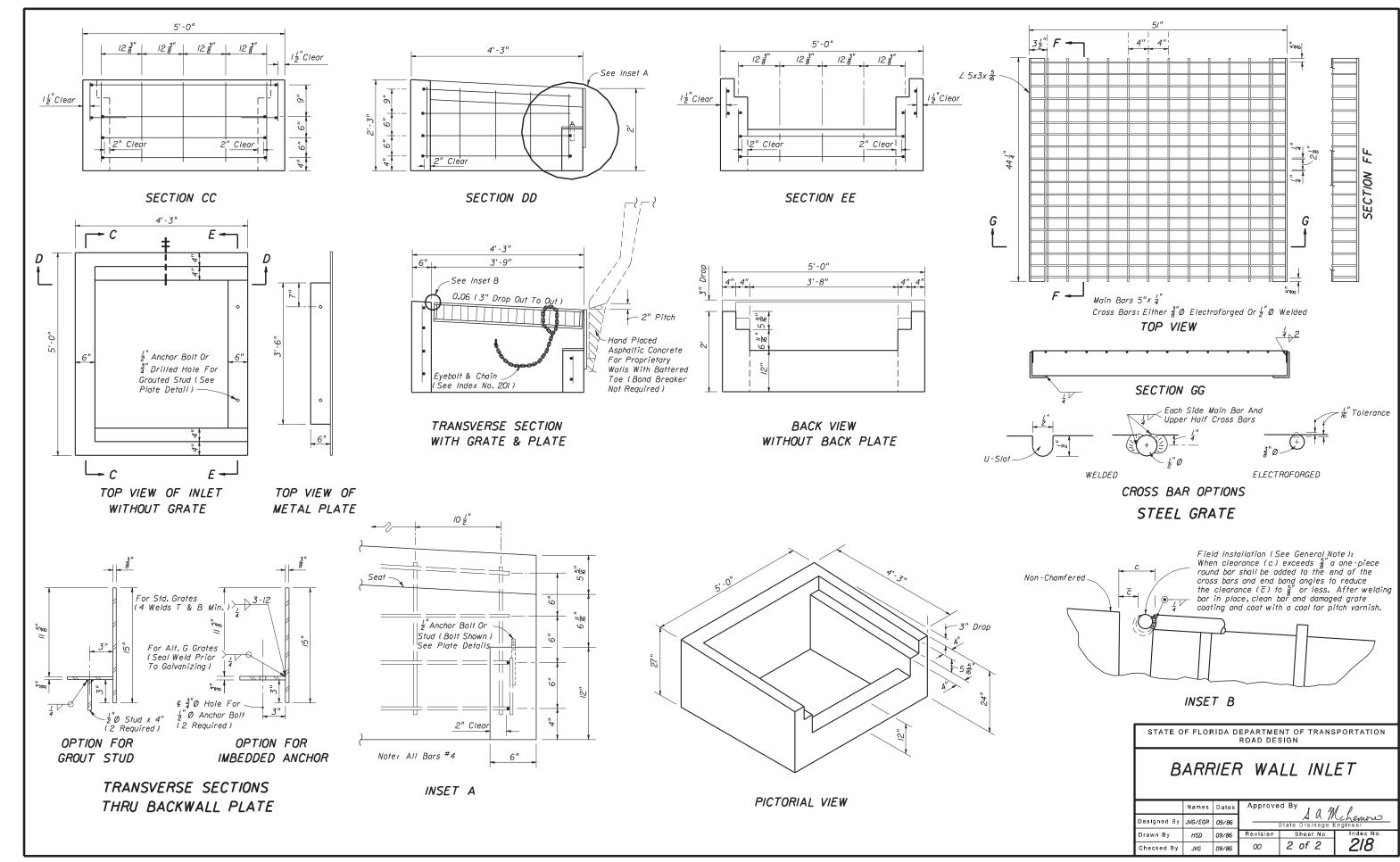


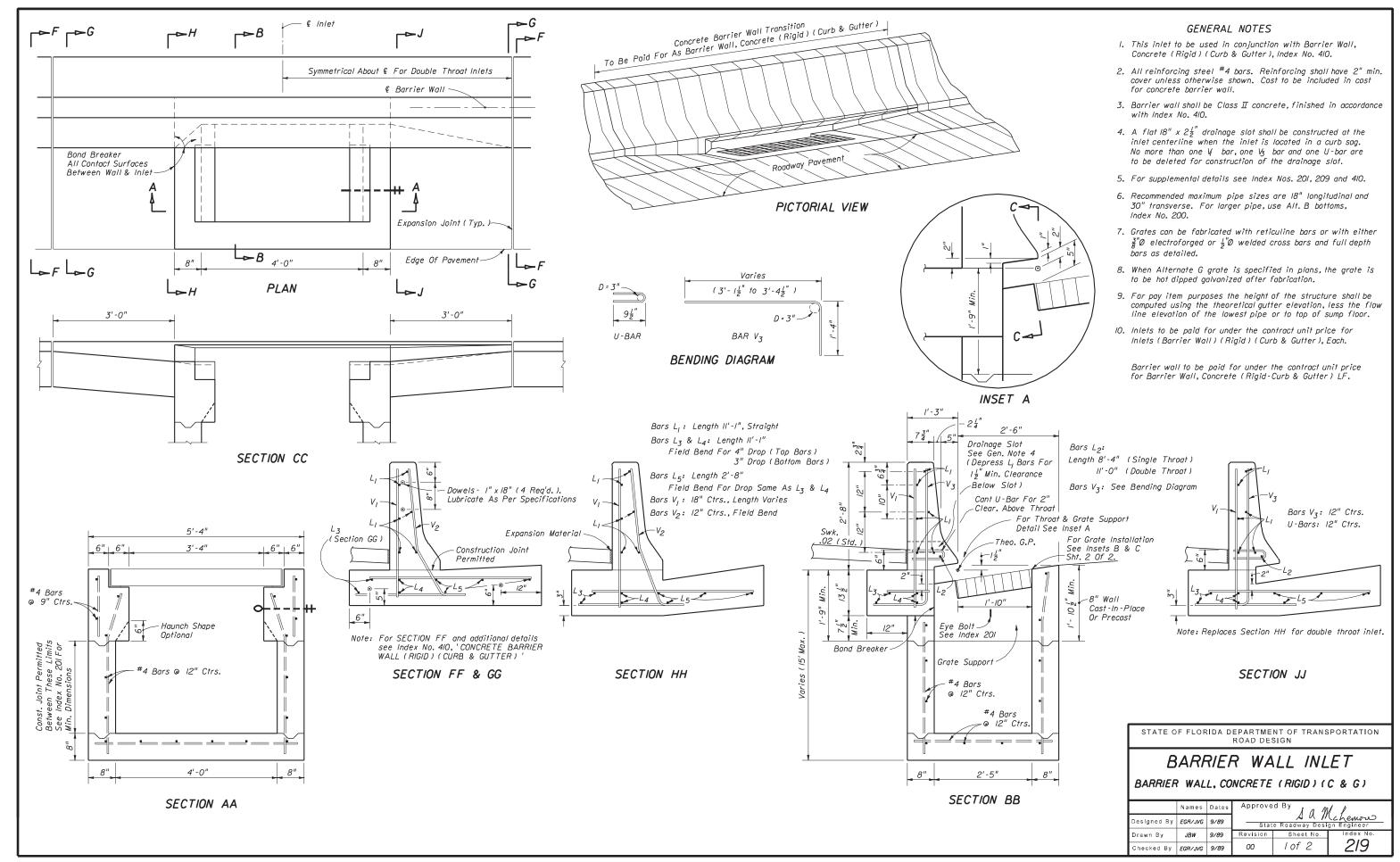
TOP VIEW (WITHOUT GRATE)

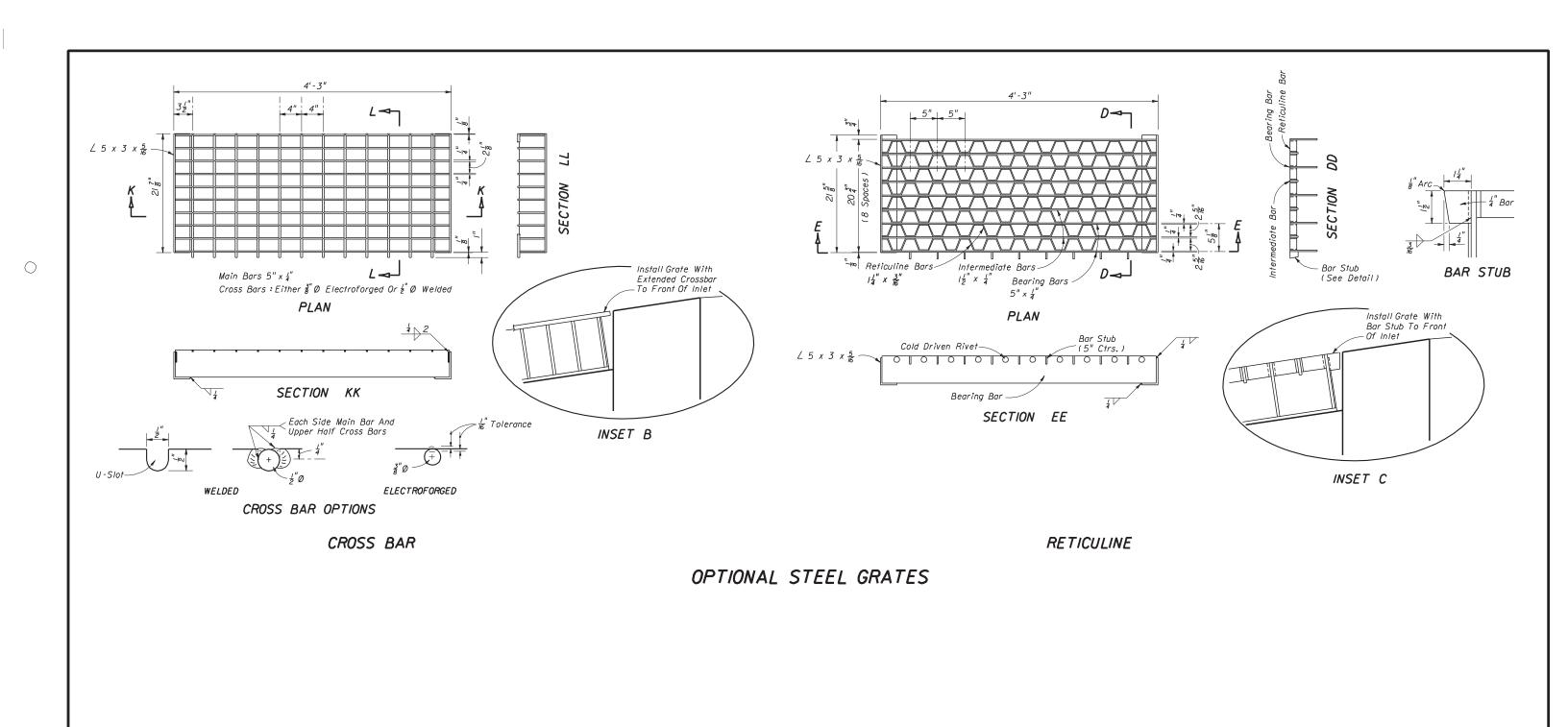
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

BARRIER WALL INLET

	Names	Dates	Approve	· 1 / //	1 0
Designed By	JVG/EGR	09/86		// ۱۸ اکر State Drainage E	(chemoro)
Drawn By	HSD	09/86	Revision	Sheet No.	Index No.
Checked By	JVG	09/86	00	1 of 2	218



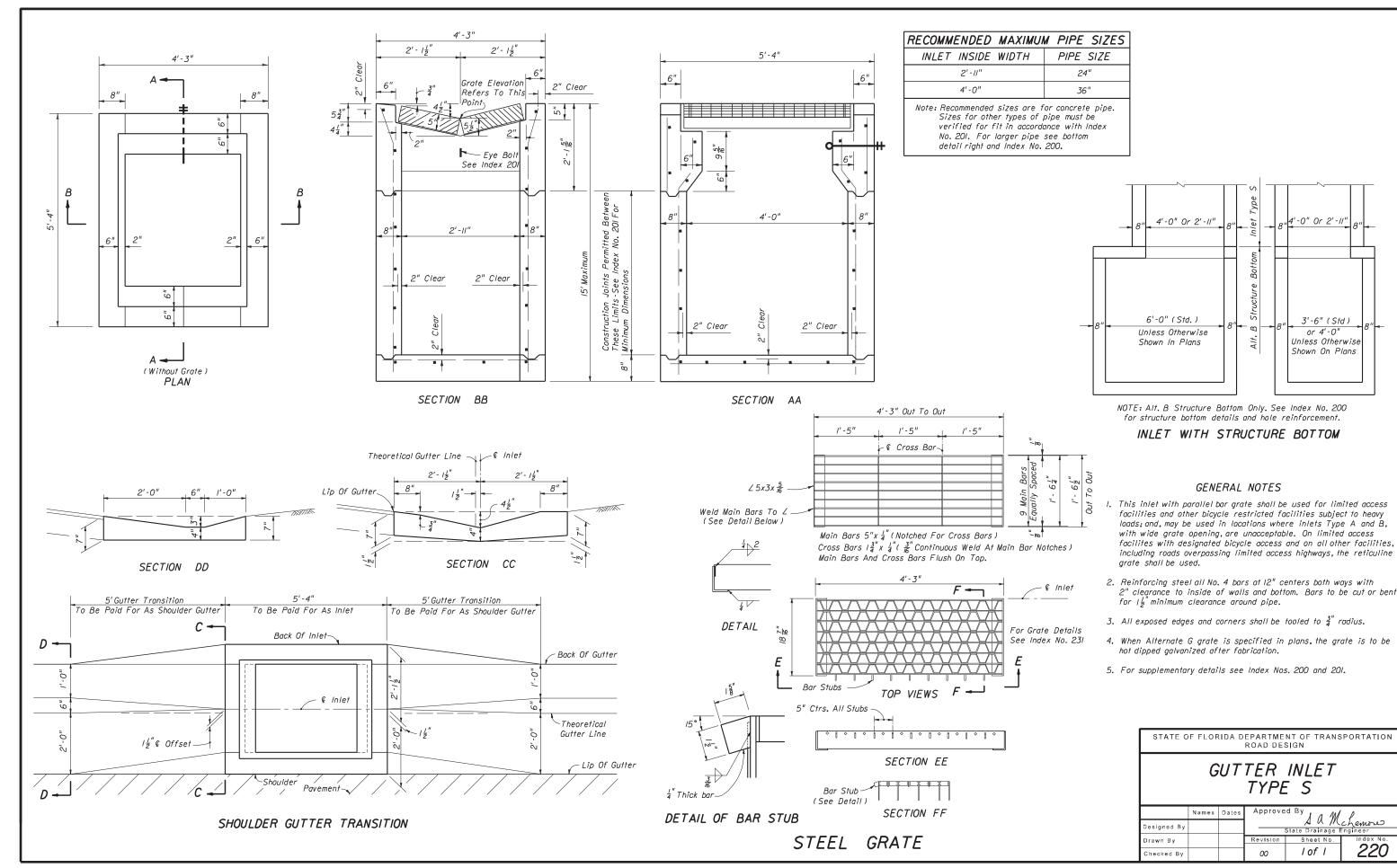


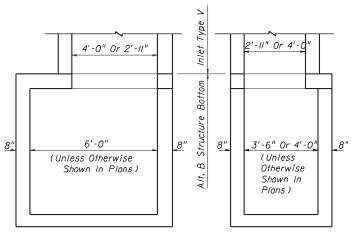


STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

BARRIER WALL INLET
BARRIER WALL, CONCRETE (RIGID) (C & G)

	Names	Dates	Approve	a By I A W	1 0
Designed By	EGR/JVG	9/89		ا// لک State Drainage E	(chemore)
Drawn By	JBW	9/89	Revision	Sheet No.	Index No.
Checked By	EGR/JVG	9/89	00	2 of 2	219



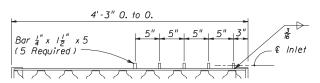


NOTE: Alt. B Structure Bottom Only. See Index No. 200 for structure bottom details and hole reinforcement. (For Pipes 30" Dia. And Larger)

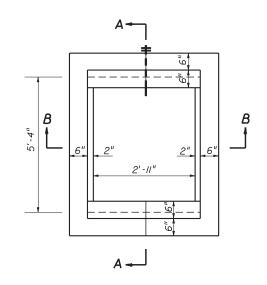
INLET WITH STRUCTURE BOTTOM

GENERAL NOTES

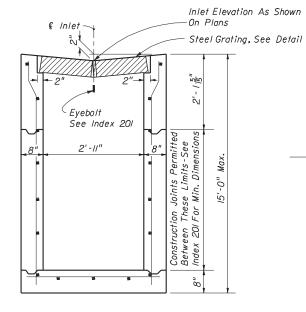
- This inlet is designed for village swales, ditches, or other areas subject to heavy wheel loads, minimum debris and subject to pedestrian and/or bicycle traffic.
- 2. When alternate "G" grate is specified in plans, the grate is to be hot dipped galvanized after fabrication.
- 3. Reinforcing #4 bars at I2" ctrs. both ways. Cut or bend bars out of way of pipe to clear pipe $l_{\perp}^{L''}$.
- 4. All exposed edges and corners shall be tooled to $\frac{1}{4}$ radius.
- 5. Recommended maximum pipe sizes shown are for concrete pipe.
- 6. For supplementary details see Index No. 201.



OPTIONAL BAR SPACING





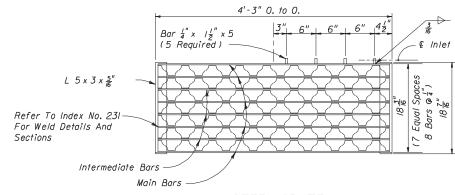


SECTION BB Recomended Maximum Pipe Size:
2'-II" Wall - 24" Size

4'-0" Wall - 36" Size

2" Clear 2" Clear 2" Clear

SECTION AA



STEEL GRATE

TWO REQUIRED PER INLET

5" Steel Grate

Main Bars 5" x ¼"

Intermediate Bars I ½" x ¼"

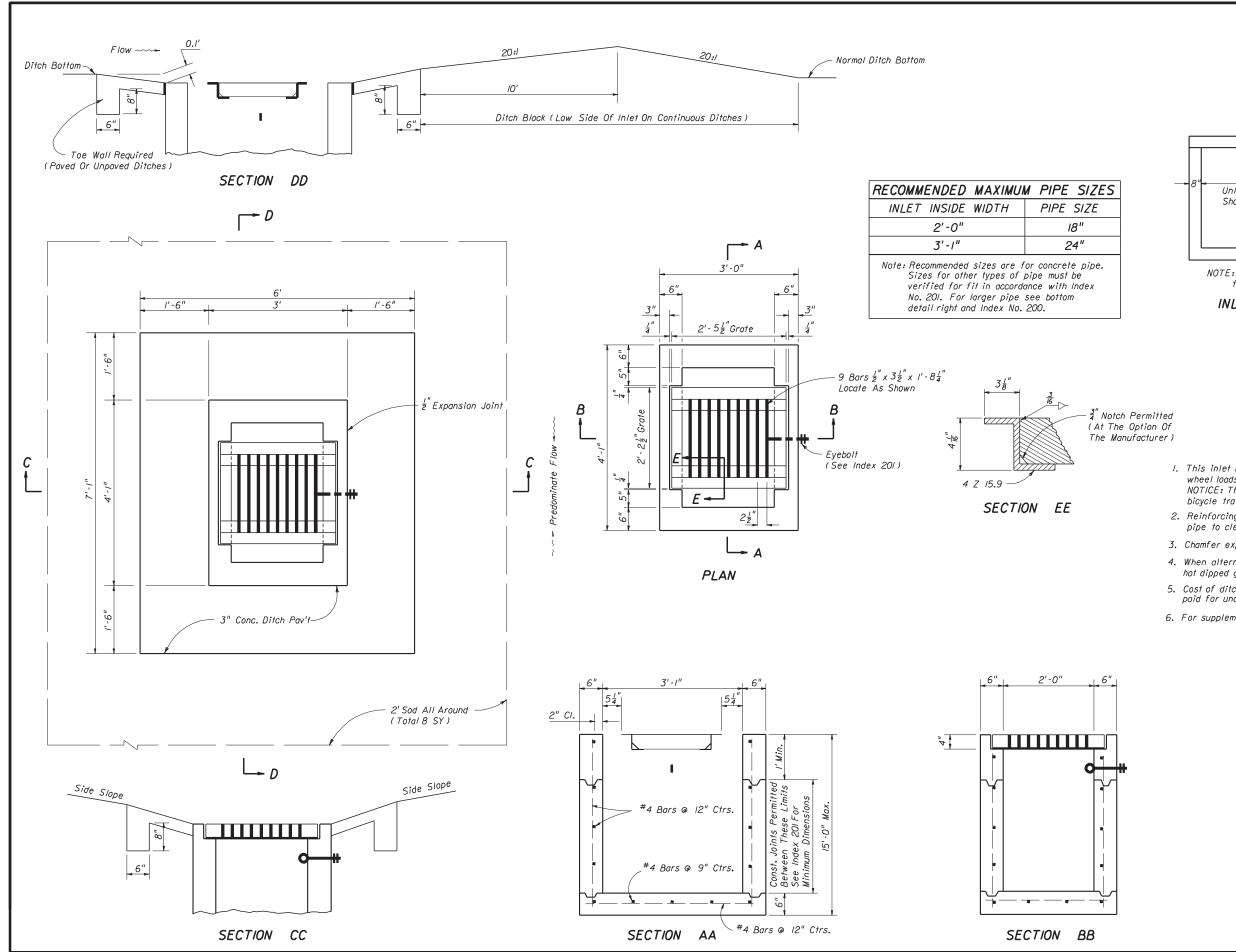
Reticuline Bars I4"x 3"

Steel Grate: Manufactured By Borden, Florida Steel, U.S. Foundry Irving, Reliance, Greulich (Or Equal).

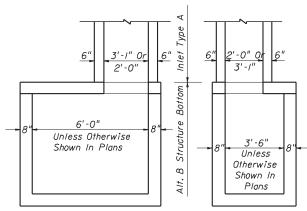
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

GUTTER INLET TYPE V

	Names	Dates	Approve	d By	10
Designed By					chemous age Engineer
Drawn By	WHW	4/57	Revision	Sheet No.	Index No.
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NOTE: Alt. B Structure Bottom Only. See Index No. 200 for Structure Bottom Details And Hole Reinforcement.

INLET WITH STRUCTURE BOTTOM

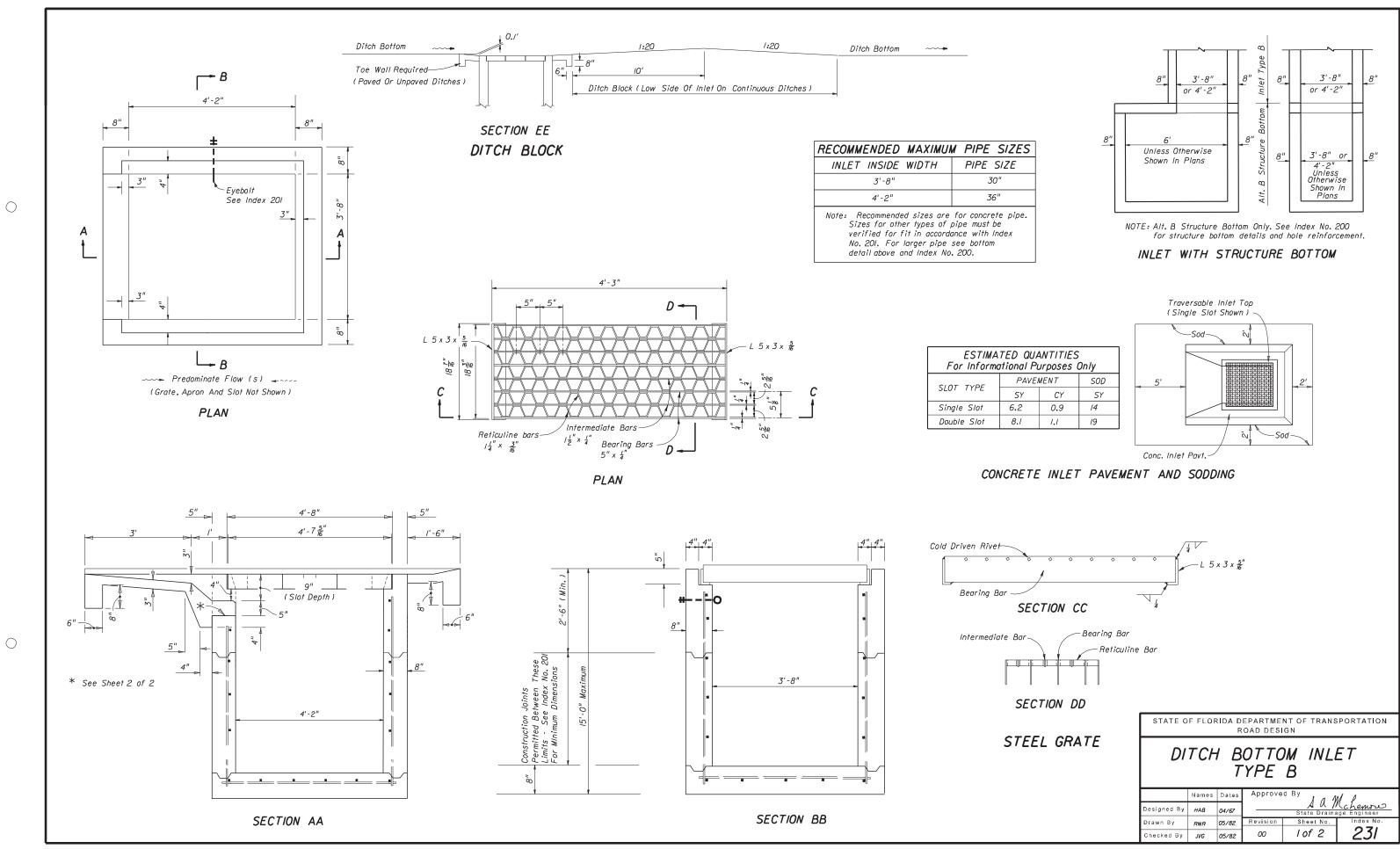
GENERAL NOTES

- I. This inlet is designed for ditches, medians, or other areas subject to heavy wheel loads on limited access facilities where debris may be a problem. NOTICE: This inlet is not for use in areas subject to pedestrian and/or bicycle traffic.
- 2. Reinforcing-2" clearance to inside face. Cut or bend bars out of way of pipe to clear pipe by l_{z}^{H} .
- 3. Chamfer exposed edges ($\frac{3}{4}$ Chamfer).
- 4. When alternate "G" grate is specified in plans, the grate is to be hot dipped galvanized after fabrication.
- 5. Cost of ditch paving to be included in the cost of Inlet. Sodding to be paid for under contract unit price for Sodding, SY.
- 6. For supplemental details see Index No. 201.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

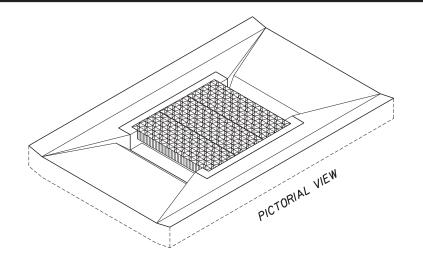
DITCH BOTTOM INLET TYPE A

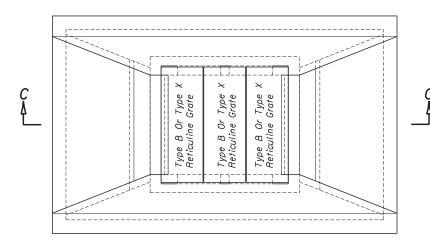
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æ : PLAN 3 Type B Or SECTION AA SINGLE SLOT

SECTION BB





PLAN

SECTION CC

DOUBLE SLOT

- Inlet Box (Line Type Indicates Existing Box To Facilitate Depiction Of Partial Construction On Existing Inlets)
- * On new boxes the traversable top may be cast as a monolithic unit or cast in segments, and the location of this line may be lower to facilitate handling and placement; however, the slot depth is to remain at 9 inches. See Index No. 201 for top to wall connection. For converting to traversable tops on existing inlets remove concrete to this line and expose the existing reinforcement. Reshape or splice in reinforcement to penetrate the rim and returns of the grate seat, and bend the reinforcement into the slot shelf to extend into the abutting throat pavement.

TRAVERSABLE TOPS FOR INLETS TYPE B AND FOR CONVERSIONS OF EXISTING INLETS TYPE B AND TYPE X

GENERAL NOTES

- I. The general purpose of the inlet top designs are:
 - For ditches, medians or other areas subject to heavy wheel loads accommodating minimal debris locations and debris improsed locations.
 - b. Provide full grate and horizontal slot designs for new construction.
 - c. Provide full grate and horizontal slot designs for replacing the verticle slot tops on existing Inlets Type B and Type X that are in locations which have become pedestrian active.
- 2. Box, walls and bottoms reinforcing steel all #4 bars at l'centers both ways with 2" clearance to inside of walls and bottom. Bars to be cut or bent for l'\(\frac{1}{2}\)" minimum clearance around pipe.
- 3. When Alternate G grates are specified in the plans, the grates are to be hot-dipped galvanized after fabrication.
- 4. Cost for constructing traversable tops on new inlet boxes shall be included in the contract unit price for Inlets (DT BOT) (Type B), EA., and shall include the cost for surrounding payement.
- Existing Inlets Type B and Inlets Type X that are converted to traversable inlet tops shall be paid for under the contract unit price for Inlets (DT BOT) (Type B) (Partial), EA. Unit price and payment shall be full compensation for inlet conversion and shall include the removal and disposal of any existing inlet pavement; the removal and stockpiling or disposal of sufficient material from the existing inlet box to facilitate construction of the required inlet top; construction of the required inlet conversion; backfill construction; construction of inlet pavement; reusing, supplementing, transferring or replacing grates as required by plans or as directed by the Engineer; any required earthwork for ditch restoration within 30' of the inlet; and, seeding and mulching disturbed grasses.
- 5. Ditch povement shall be paid for, separate from the inlet, by povement types and units as called for in the plans.
- 6. Sod will be paid for under the contract unit price for Sodding, SY.
- 7. For supplementary details see Index No. 201.

DESIGN NOTES

I. The type of top (single or double slots) depends on the approach ditch configuration and the hydraulic requirements of the site. The designer will stipulate in the plans the type of top to be constructed at each individual inlet location.

On existing inlets conversion grates shall be constructed at the original grate elevations unless other elevations are called for in the plans. When plans call for the inlet top to be constructed to support storm water detention, details for ditch modifications and underdrains shall be shown in the plans.

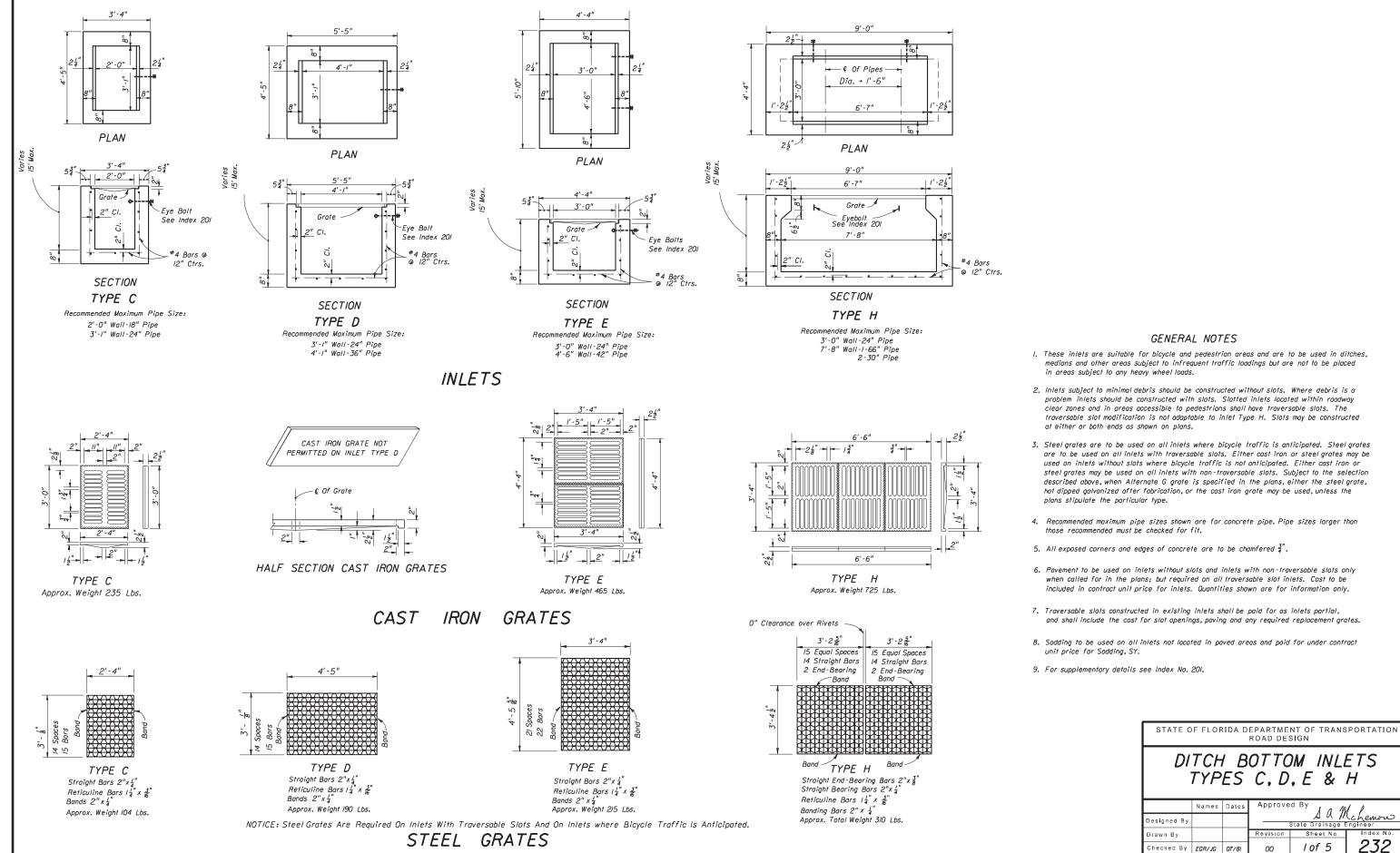
MAINTENANCE NOTES

 Traversable inlet tops that are constructed by maintenance contract or by maintenance forces may reuse the existing grates that are determined by the Maintenance Engineer to be functionally sound, and their reuse is so directed by the Maintenance Engineer. Existing grates approved for reuse and new grates may be mixed, matched or replaced as directed by the Maintenance Engineer.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

DITCH BOTTOM INLETS TYPE B

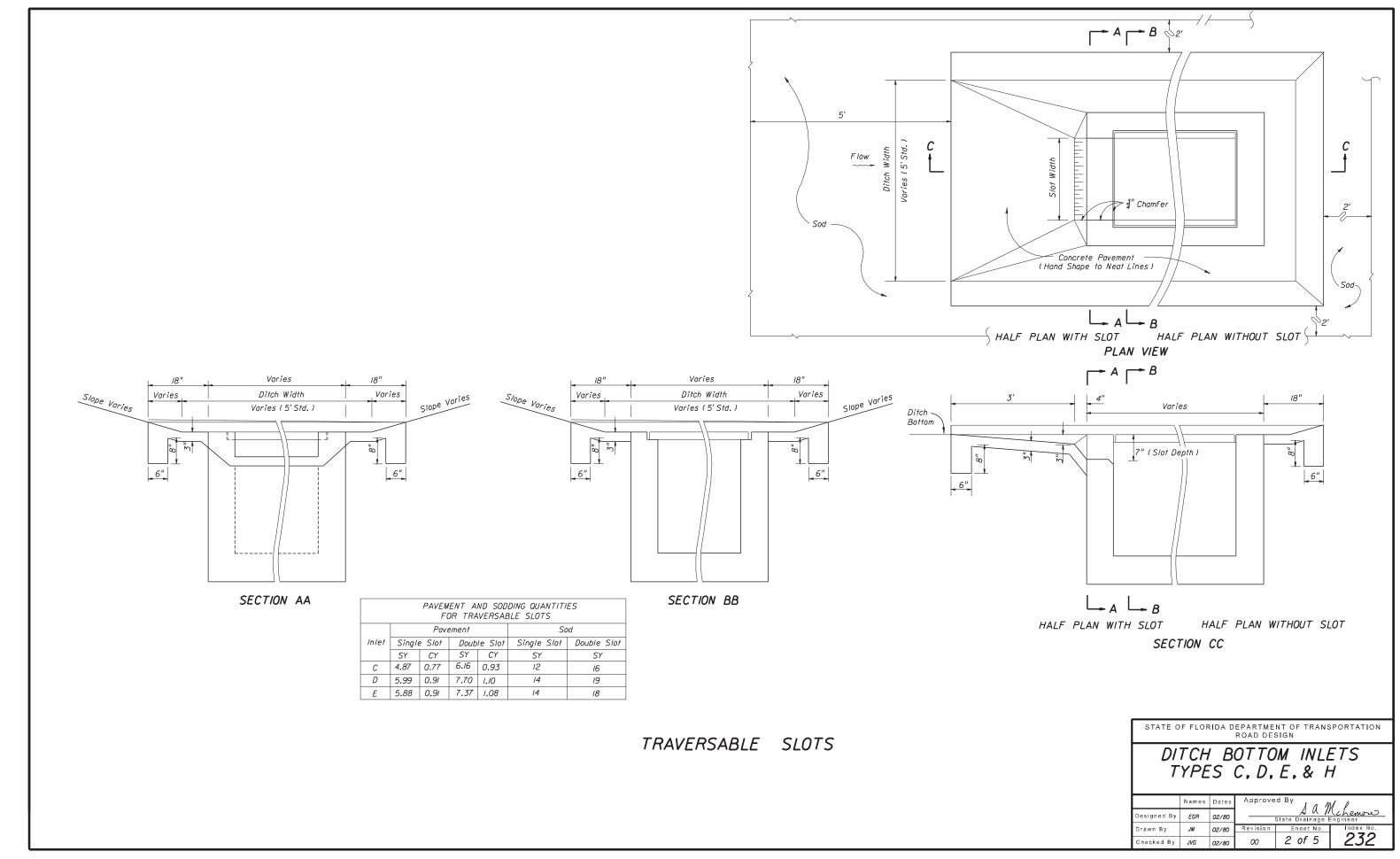
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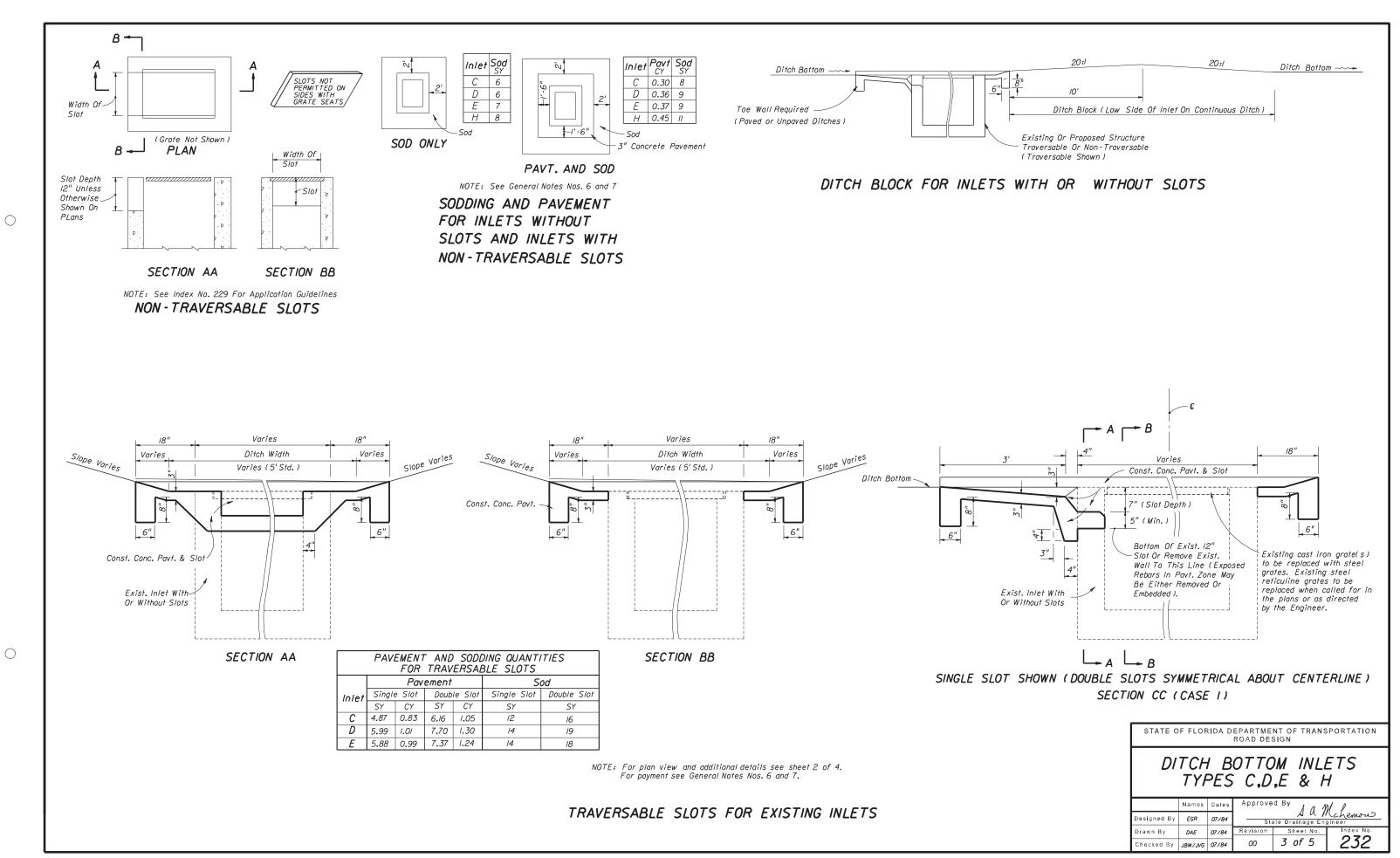


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DITCH BOTTOM INLETS TYPES C, D, E & H

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Existing cast iron grate to be replaced with steel grate. Existing steel reticuline grate to be replaced when called for in the plans or as directed by the Engineer. Break Angle Not To Exceed 3° (1:20) Exist. Ditch Bottom Remove This Portion Of (Box With Slot Shown) Varies (See Plans) Box And Construct New Top, Seats And Slot(s) Const. Conc. Pavt. & Slot Bottom Of Transition Ditch 7" (Slot Depth) Grate -5" (Min.) These Points Can Be The 6" Same Or Nearly The 6" Same Where Ditch Remove Exist. Wall To Grades Are Flat. This Line (Exposed Rebars In Pavt. Zone May Be Either Removed Or Imbeded) Exist. Inlet With Or Without Slots SINGLE SLOT SHOWN (DOUBLE SLOTS SYMMETRICAL ABOUT CENTERLINE) Break Angle Not To Exceed 3° (1:20). SECTION CC (CASE 2) Minimum Slope For Transition Ditch 0.10% Bottom Of Transition Ditch; Or Unless Existing Ditch Bottom Of Exist. Ditch For Is Flatter. Exist. Inlet Without Slot Const. Conc. Pavt. & Slot Ditch Grade For Detention Concept -7" (Slot Depth) -5" (Min.) Bottom Of Exist, Ditch For Bottom Of Exist. 12" Slot Exist. Inlet With Slot Existing cast iron grate to be replaced Or Remove Exist. Wall To with steel grate. Existing steel reticuline This Line (Exposed Rebars grate to be replaced when called for in In Pavt. Zone May Be Either the plans or as directed by the Engineer. Slope Same As Exist. Ditch-Removed Or Imbeded) Exist. Inlet With Underdrain, Type I Or Without Slot To Be Constructed Only With Detention Ditch *Extra Pavement When Exist. Concept, And Only When Called For In The Plans Slot Exceeds 12" In Depth SINGLE SLOT SHOWN (DOUBLE SLOTS SYMMETRICAL ABOUT CENTERLINE) SECTION CC (CASE 3)

TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

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DESIGN NOTES FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- The general purpose of these conversions is to remove the hazard of the protruding inlet top, while not creating a hazard by depressing the top too deeply.
- 2. The corrective procedure depends on the approach ditch grade and hydraulic requirements of the site. The selection of the appropriate case depends on the relationship between inlet top and ditch elevation, and, on the vertical clearance between the top of the uppermost pipe(s) and the grate. The purpose for the Case I conversion is to add the traversable slot to an existing inlet where top removal, change in grate elevation and ditch transitions are not required. Case 2 will normally be applicable to ditches with flatter grades adjoining the inlet. Case 3 will normally be applicable to ditches with steeper grades adjoining the inlet where buildup of the existing ditch is acceptable.
- 3. The designer shall stipulate in the plans which case is to be constructed at each individual inlet location,

Where the existing inlet top is above the existing ditch (Case 2) but borrow material will be required to adjust the ditch (Case 3), and vertical clearance or other conditions do not prevent removal of the inlet top, the designer should call for Case 2. The designer shall determine if ditch reconstruction is required more than 35 feet beyond any traversable slot side and shall include separate pay items in the plans to cover the cost for that portion of required ditch reconstruction exceeding the 35 foot limit. The designer shall also determine whether ditch pavement is required for ditch restoration within the 35 foot limit and include that povement under a pay item separate from the inlets partial.

When the detention ditch concept is to be used with Case 3, the designer shall stipulate 'Case 3 (Detention)' in the plans.

The designer shall determine whether tight soil or other conditions at each individual inlet indicates the need for underdrain in Case 3 conversions and shall call for Underdrain, Type I in the plans.

METHOD OF PAYMENT FOR TRAVERSABLE SLOT INLETS (PARTIAL) FOR EXISTING INLETS

- I. Existing inlets converted to traversable slot tops under Cases I, 2 and 3 shall be paid for as inlets partial, each. Case shall not be included in the pay item description.
- 2. All ditch reconstruction work within 35 feet of each traversable slot conversion, whether required by these details or as a direct result of the conversion, shall be included as a part of the partial cost. Reconstruction work shall include excavation and removal of surplus materials or borrow materials in place, grading, compaction, shaping and seeding and mulching. Sodding, ditch pavement and underdrain are not included as part of the inlet partial cost and are to be paid for separately.
- Inlet pavement and sodding shall be in accordance with the sections on this detail and with the Plan on Sheet 2 and Sections AA, BB and CC (as Case I) and tabular quantities on Sheet 3.
- 4. Unit price and payment shall constitute full compensation for inlet conversion, replacement grate(s) ditch reconstruction, seeding and mulching, and shall be paid for under the contract unit price for Inlets (DT Bot) (Type ___) (Partial), each.

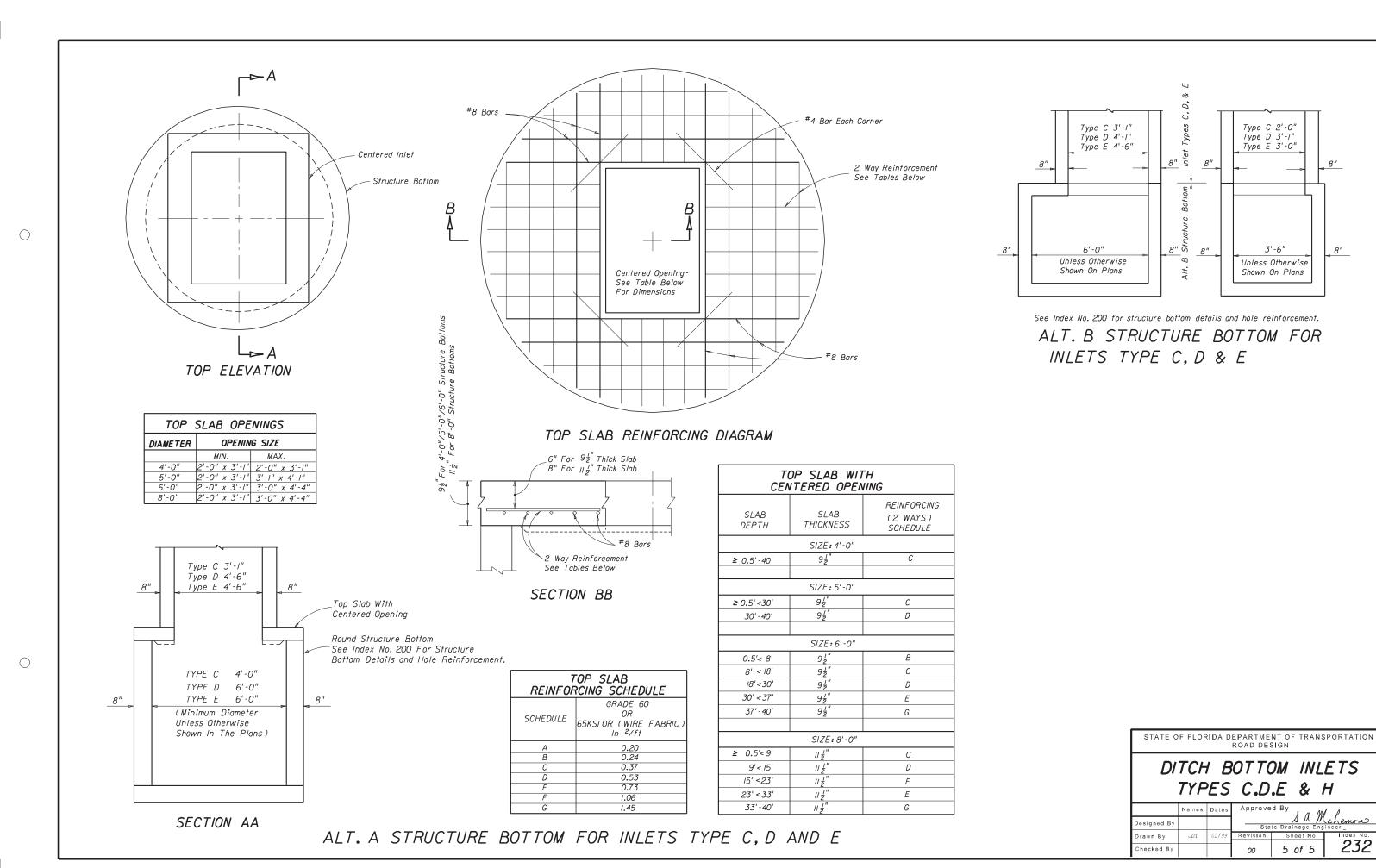
Sodding shall be paid for under the contract unit price for Sodding, SY.

Ditch pavement shall be paid for separate from the inlet by pavement type(s) and unit(s) as called for in the plans.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

DITCH BOTTOM INLETS TYPES C, D, E & H

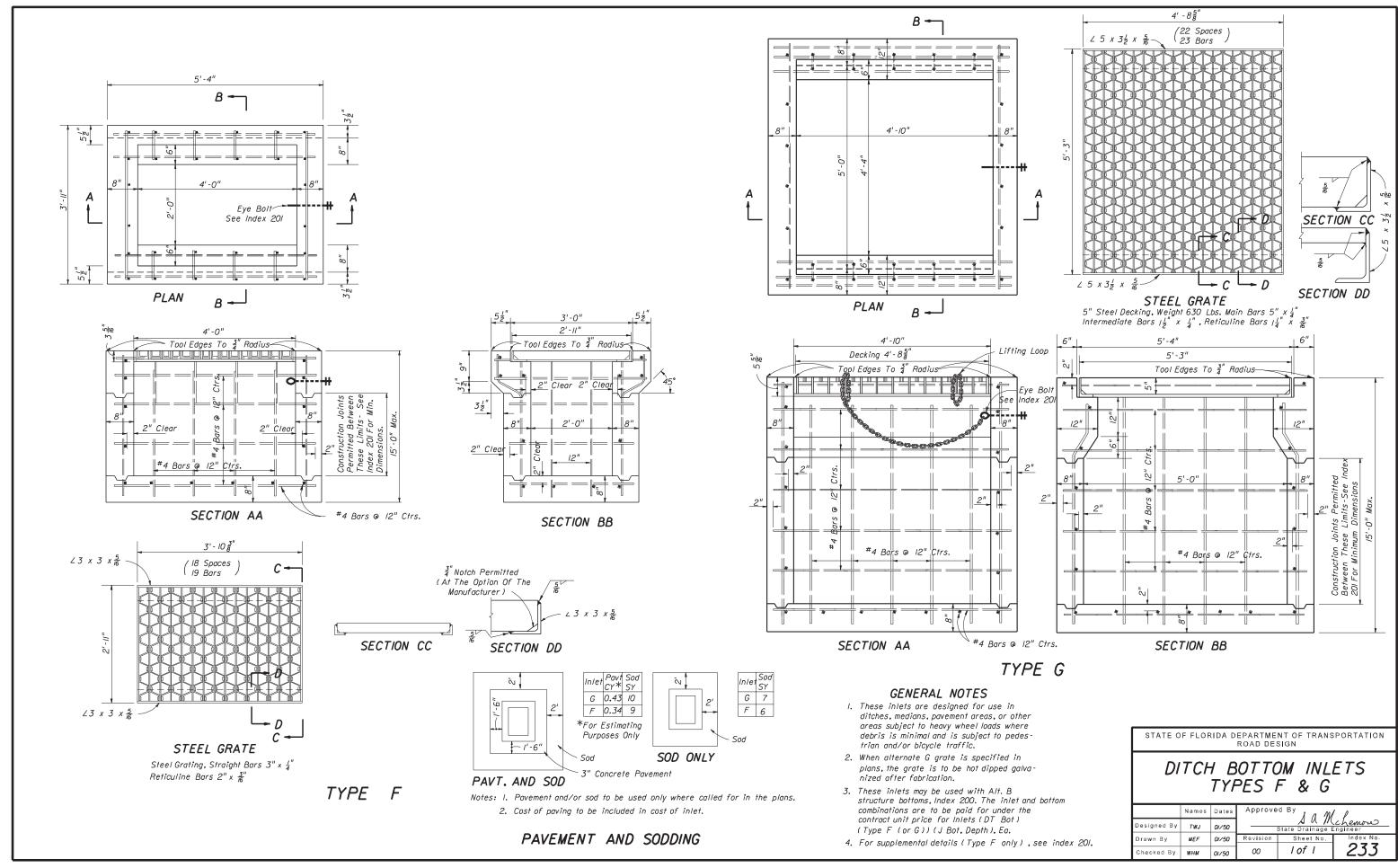
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Drawn By	HSD/dds	5/20/86	Revision	Sheet No.	Index No.
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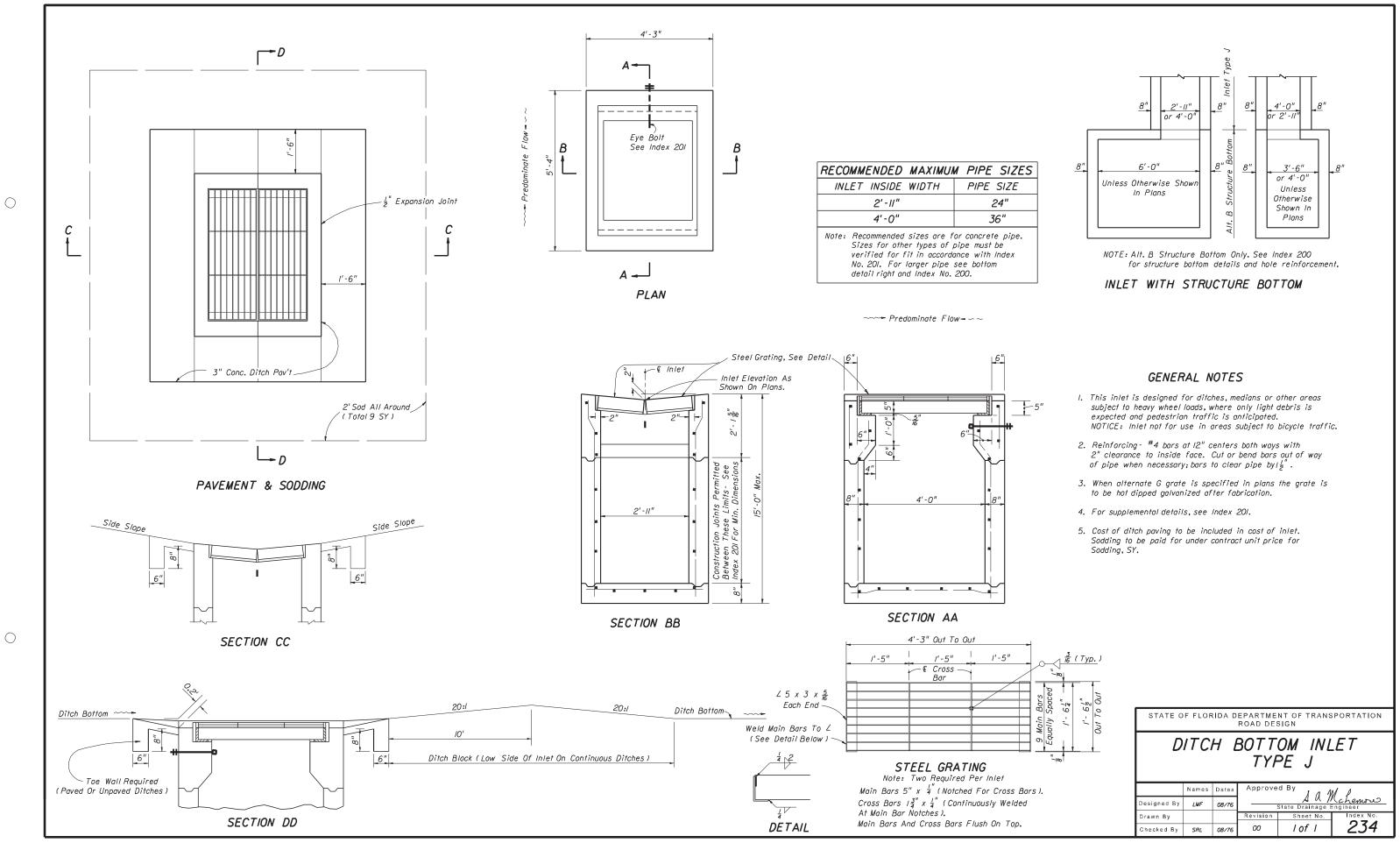


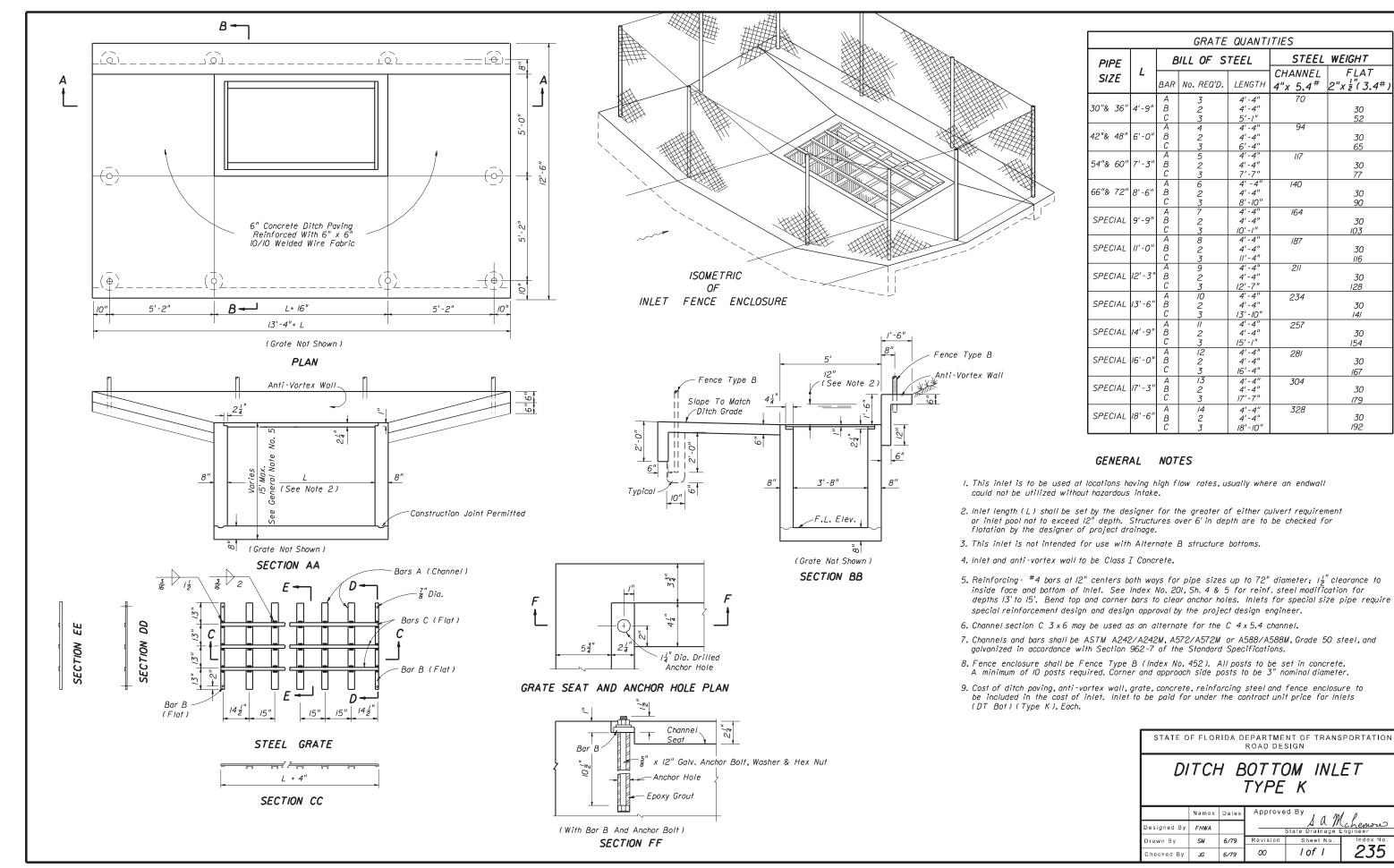
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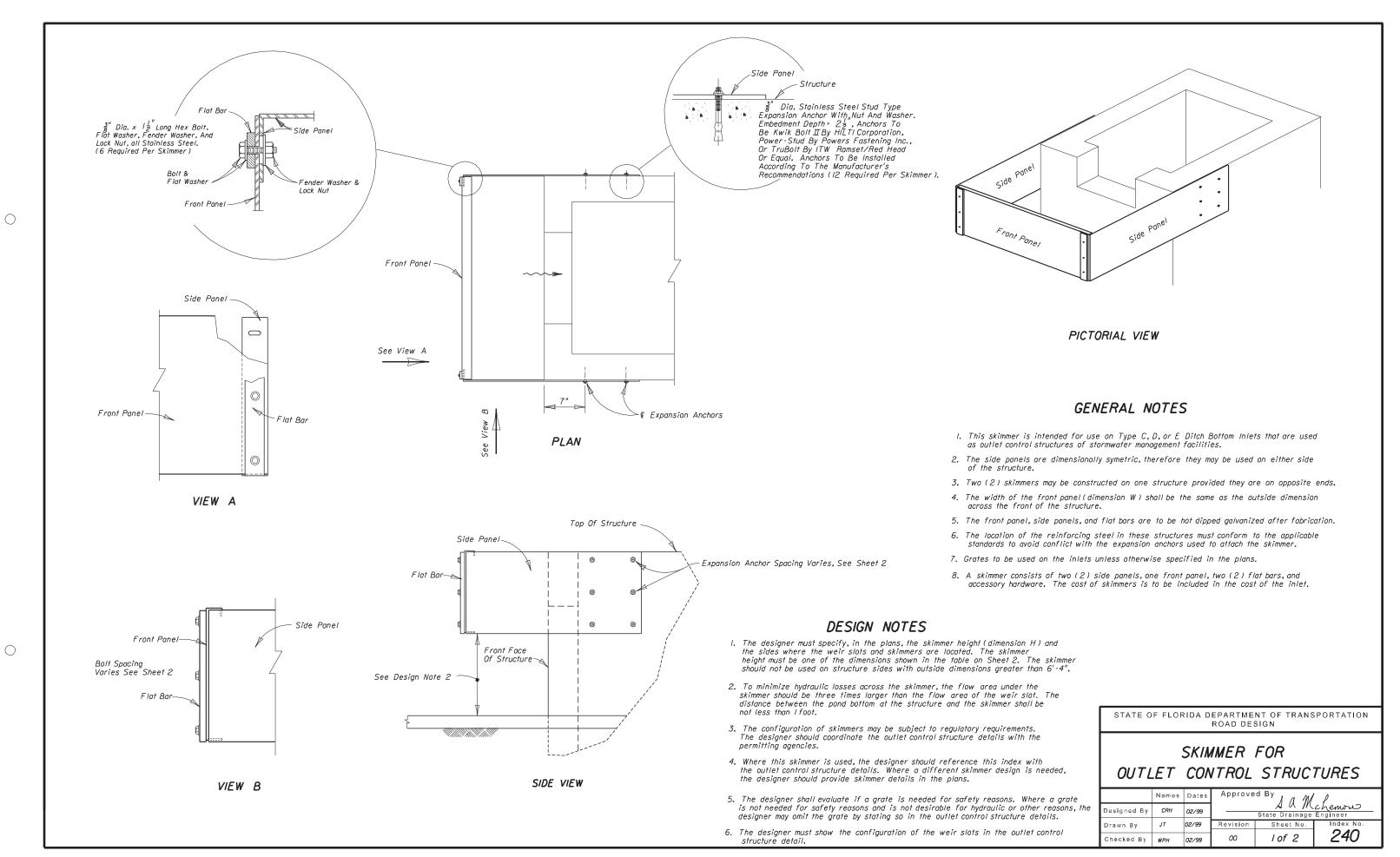
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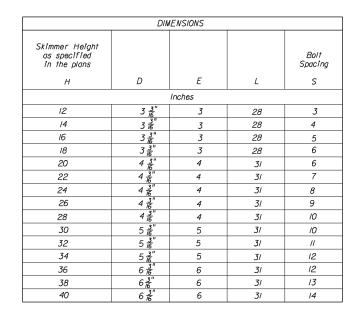


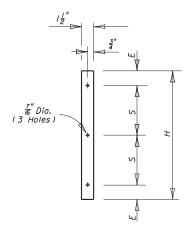




FLAT

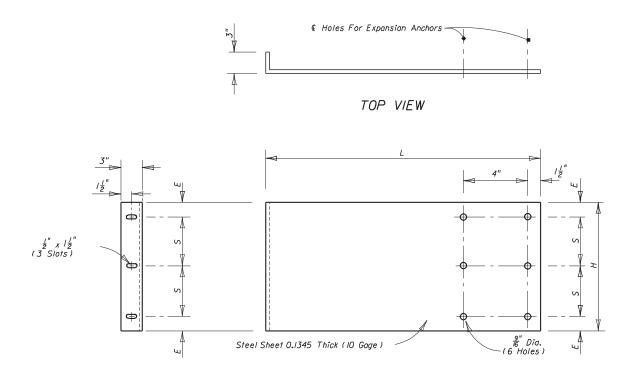






 $\frac{1}{4}$ Thick x $\frac{1}{2}$ Wide

FLAT BAR



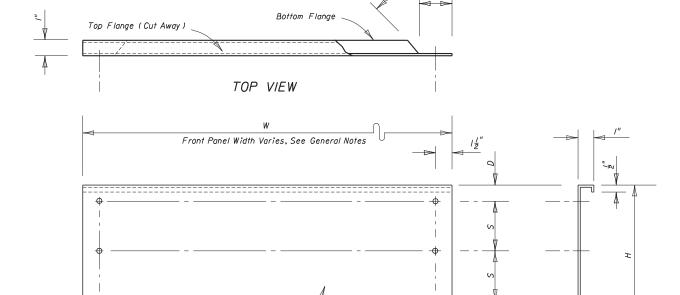
END VIEW (FRONT)

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SIDE VIEW

SIDE PANEL



FRONT VIEW

Steel Sheet 0.1345" Thick (10 Gage)

END VIEW

FRONT PANEL

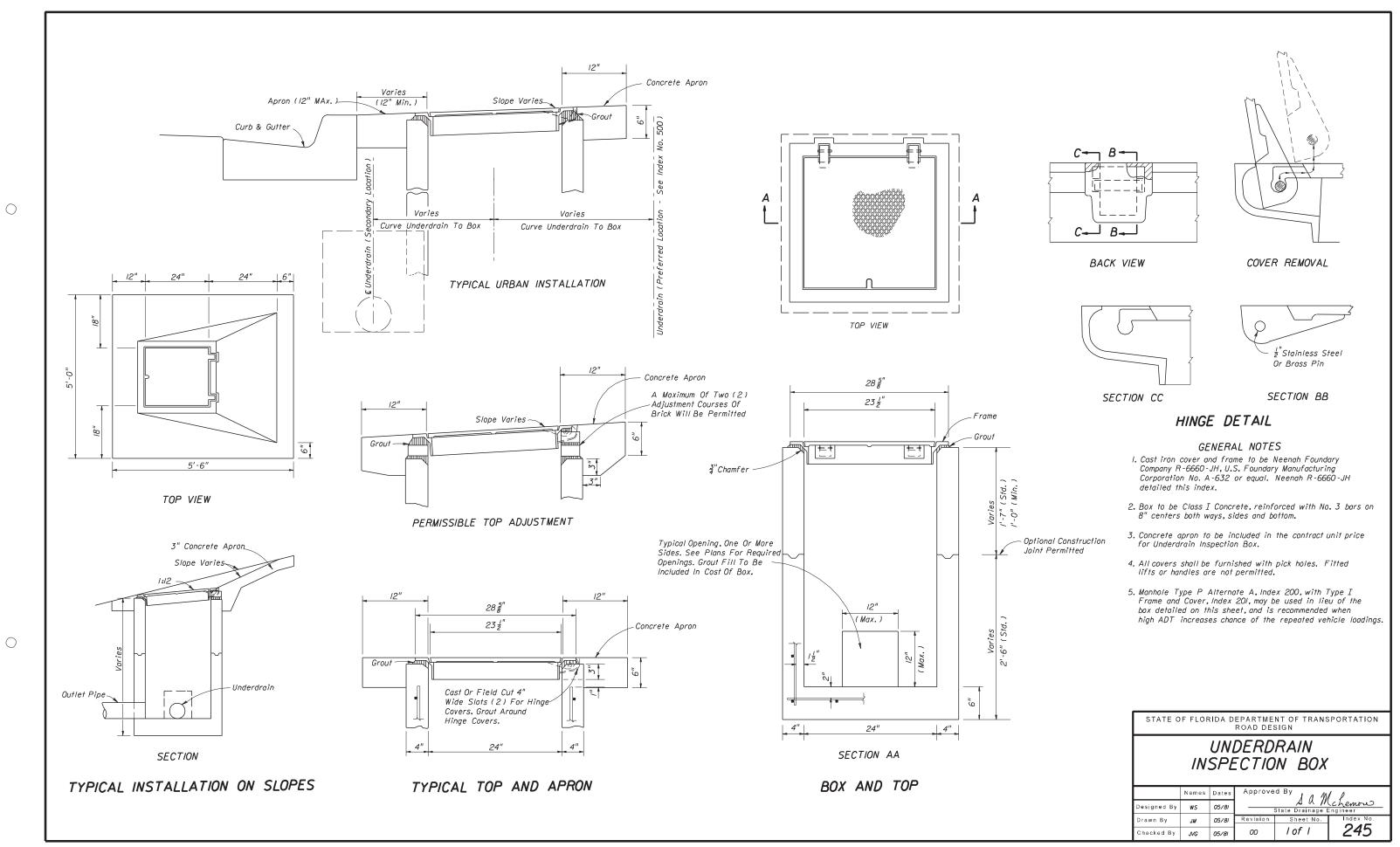
9" % Dia. (6 Holes)

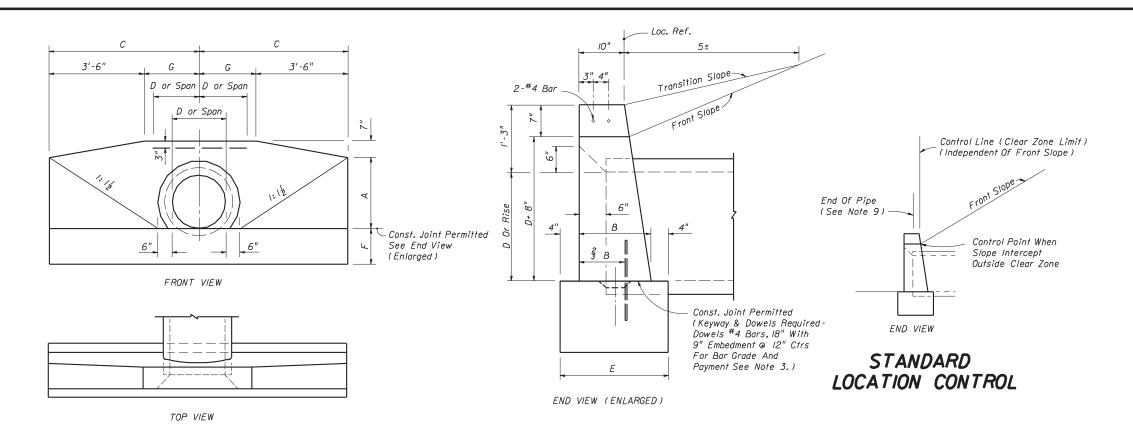
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SKIMMER FOR OUTLET CONTROL STRUCTURES

	Names	Dates	Approve	d By	W P
Designed By	CRH	02/99	Stat	ر لا لا e Roadway Desi	gn Engineer
Drawn By	JT.	02/99	Revision	Sheet No.	Index No.
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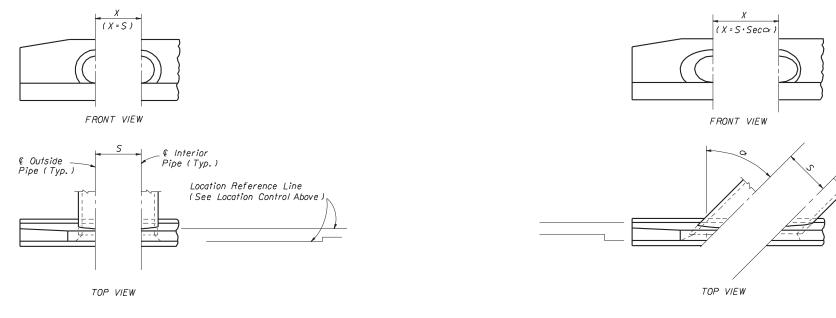
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ENDWALL DIMENSIONS (EXCLUSIVE OF MULTIPLE PIPE SPACING)

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NORMAL PIPE SKEWED PIPE

- Center To Center Pipe Spacing
- X Centerline To Centerline Dimension At Face Of Headwall

LEGEND

ENDWALL POSITIONS FOR SINGLE AND MULTIPLE PIPE AND SPACING FOR MULTIPLE PIPE

GENERAL NOTES

- Endwall dimensions, locations and positions are for round and elliptical concrete pipe and for round and pipe-arch corrugated metal pipe. Round concrete pipe shown.
- 2. Front slope and ditch transitions shall be in accordance with Index No. 280.
- 3. Endwalls may be cast in place or precast concrete. Reinforcing steel shall be Grades 40 or 60. Additional reinforcement necessary for handling precast units shall be determined by the Contractor or the supplier. Cost of reinforcement shall be included in the contract unit price for concrete, (endwalls).
- 4. All exposed corners and edges of concrete are to be chamfered $\frac{3}{4}$.
- 5. Concrete meeting the requirements of ASTM C478 (4000 psi) may be used in lieu of Class I concrete in precast items manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.
- On outfall ditches with side slopes flatter than 1:1½
 provide 20' transitions from the endwall to the flatter
 side slopes, right of way permitting.
- 7. For sodding around endwalls see Index No. 281.
- 8. Payment for concrete quantities for endwalls skewed to the pipe shall be made on the following basis:

Endwall Skew To Pipe	Use Tabulated Value
0° to 5°	O°
6° to 15°	/5°
16° to 30°	<i>30</i> °
31° or over	45°

- Pipe length plan quantities shall be based on the pipe end locations shown in the standard location control end view, or lengths based on special endwall locations called for in the plans.
- 10. Payment for pipe in pipe culverts shall be based on plan quantities, adjusted for endwall locations subsequently established by the Engineer.
- II. Endwalls to be paid for under the contract unit price for Concrete Class I (Endwalls), CY.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STRAIGHT CONCRETE ENDWALLS SINGLE AND MULTIPLE PIPE

	Names	Dates	Approve	. 1 1/1	M P
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Checked By	JBW/JVG	83	00	1 of 2	250

DATA AND ESTIMATED QUANTITIES FOR ONE ENDWALL

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				38.4																6.49	8.11	8.17	8.39	8.85	8.43	8.50	8.7	3 9.23	9.90	10.02	10.45	11.38	10.38	10.52	0.98	11.99	11.68	11.87	12.51	13.89	12.32	12.52	13.22	2 14.73	42"
48"	12.57	25.14	37.7	71 50.2	8 4'	-8"	2'-1"	9'-	6" 2	2'-9"	2'-0"	6'-0	6'-	9" 6'	-9"	7'-0"	7'-10'	9'-7	8.15	8.38	10.40	10.48	10.75	11.33	10.8	5 10.94	1 11.2	2.3 11.87	12.64	12.80	13.34	14.50	13.34	13.51	14.11	15.39	14.89	15.13	15.93	17.68	15.82	16.08	16.97	7 18.90	48"
54"	15.90	31.80	47.7	0 63.6	0 5'	-2"	2'-6"	10'-6	6" 3	'-2"	2'-3"	7'-0	7'-	<i>9"</i> 7'	-8"	7' -	8'-10	" 10' -10	" //.7/	11.77	15.23	15.35	15.78	16.69					18.77	19.02	19.86	21.69					22.29	22.66	23.9	26.67					54"

											CO	RRU	GATL	ED N	VETA	AL P	PIPE AF	CH														
		Ομ	enin	g Ar	ea															CI	ass .	I Co	ncre	te (CY)							Approx.
			(SF)		Dimensions													ber (Of P	ipe .	And	Skei	w Ai	ngle	Of I	Pipe	•		Snan	Rise	Equiv. Round
Span	Rise	Nun	ber (Of Pi	pes	A	В	С	F	F	G	· ·			Χ		Single		Doul	b/e			Tri			C	luadi	ruple	•	Spun	11136	Pipe
		1	2	3	4	<u> </u>				_ ′			o°	15°	<i>30</i> °	45°	0°	0°	<i>15</i> °	<i>30</i> °	45°	0°	<i>15</i> °	<i>30</i> °	45°	O°	15°	<i>30</i> °	45°			1 Ipc
/7"	/3"	1.1	2.2	3.3	4.4	1'-9"	1'-2"	3'-10"	1'-10"	1'-2"	0'-4"	2'-6"	2'-6"	2'-7"	2'-11"	3'-6"	1.16	1.47	1.48	1.52	1.60	1.78	1.80	1.88	2.04	2.09	2.12	2.23	2.48	17"	13"	15"
21"	15"	1.6	3.2	4.8	6.4	/'-//"	1'-2"	4'-3"	1'-10"	1'-2"	0'-9"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.33									2.40				21"	15"	18"
28"	20"	2.8	5.6	8.4	11.2	2'-4"	/'-3"	5'-2"	/'-//"				3'-5"				1.78	2.31	2.33	2.39	2.53	2.83	2.87	2.99	3.26	3.36	3.42	3.60	4.01	28"	20"	24"
35"	24"	4.3	8.6	12.9	17.2	2'-8"	1'-4"	5' -11½"	2'-0"				4'-0"	4'-2"	4'-7"	5'-8"	2.34	3.03	<i>3.05</i>	3.14	3.32	3.72	3.77	3.93	4.29	4.40	4,47	4.72	5.25	<i>3</i> 5"	24"	30"
42"	29"	5.9	11.8	17.7	23.6	3'-/"	/'-5"	6' -10½"	2'-1"	/'-5"	3'-4½"	4'-9"	4'-9"	4'-//"	5'-6"	6'-9"	3./3	4.06	4.09	4.20	4.45	4.99	5.06	5.28	5.76	5.93	6.03	6.36	7.09	42"	29"	36"
49"	33"	8.4	16.8	25.2	33.6	3'-5"	1'-6"	7'-8"	2'-2"	1'-6"	4'-2"	5'-6"	5'-6"	5'-8"	6'-4"	7'-9"	3.83	5.00	5.04	5./8	5.48	6./6	6.24	6.52	7.12	7.32	7.44	7.86	8.76	49"	33"	42"
57"	38"	10.6	21.2	31.8	42.4	3'-10"	1'-7"	8' -7날"	2'-3"	1'-7"	5'-1½"	6'-4"	6'-4"	6'-7"	7'-4"	8'-//"	4.87	6.31	6.36	6.53	6.91	7.74	7.84	8.18	9.93	9.18	9.33	9.85	10.96	57"	38"	48"
64"	43"	13.2	26.4	39.6	52.8	4'-3"	1'-8"	9'-62"	2'-4"	/'-8"	6' -0½"	7'-1"	7'-/"	7'-4"	8'-2"	10'-0"	5.88	7.64	7.70	7.91	8.37	9.40	9.52	9.94	0.86	11.15	11.33	11.97	/3.33	64"	43"	54"
71"	47"	16.9	33.8	50.7	67.6	4'-7"	1'-10"	10'-4"	2'-6"	2'-0"	6'-10"	7'-10"	7'-10"	8'-1"	9'-1"	//'-/"	7.80	10.15	10.23	10.51	11.12	12.49	12.65	13.22	4.43	14.85	15.10	15.94	17.77	71"	47"	60"

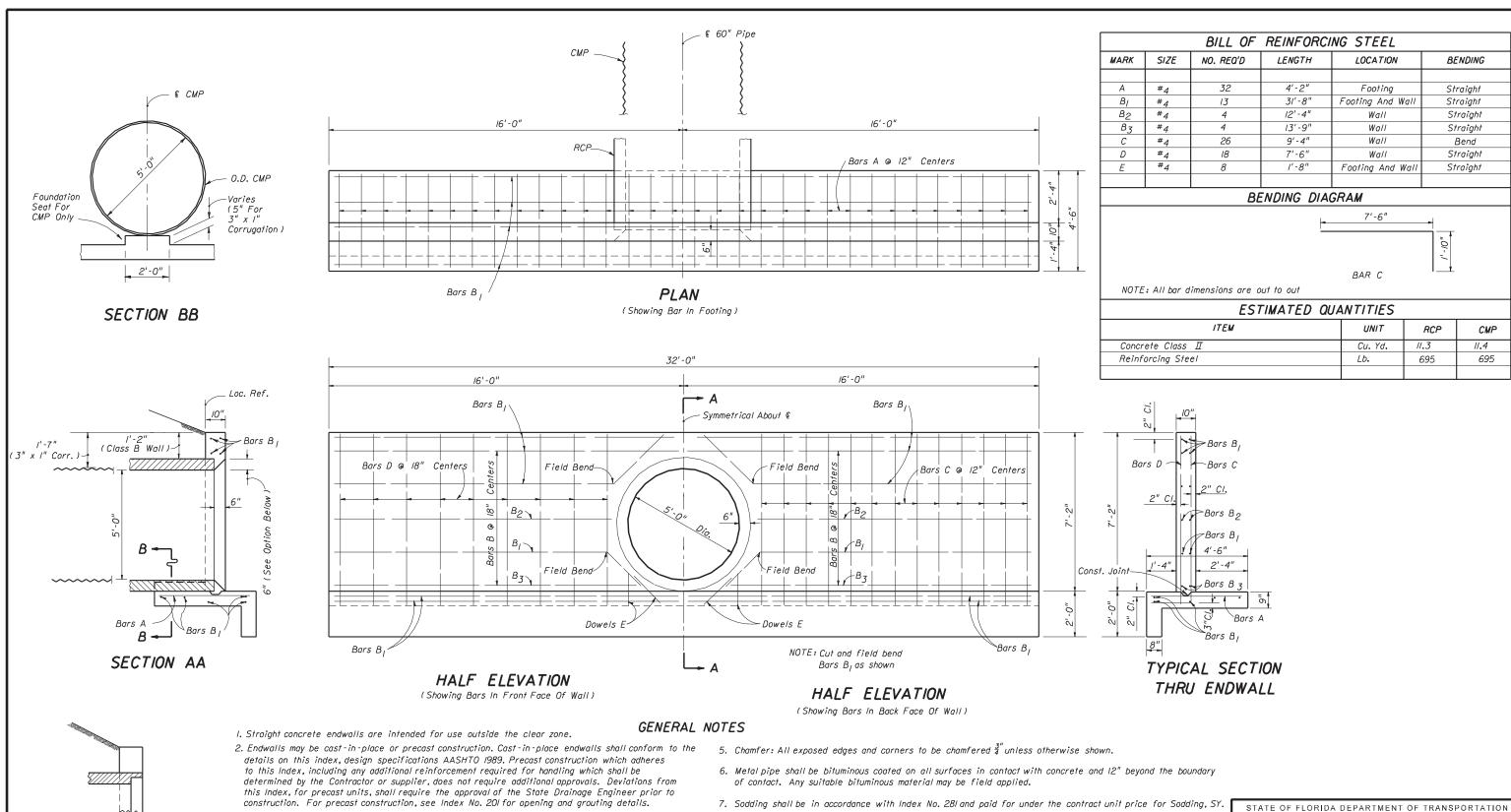
Note: Use the guidelines of General Note No. 8 for selecting tabular quantities.

												CO	ONCF	RETE	EL	LIPT	TICAL P	IPE														
		Ор	ening	g Are	ea					Dime	ensior	25								Clo	ss I	Con	cret	e ((CYJ							Approx.
			(SF	7						יוווע	.115101	15						Num	ber	Of P	ipe	And	Ske	w A	ngle	Of .	Pipe	•		Rice		Equiv. Round
Rise	Span	Nun	ber (Of Pi	pes	Ι,	В		E	E	G	S			Χ		Single		Doi	uble			Trip	ı/e		G	luadi	ruple	,	11136	Spuri	Pipe
		1	2	3	4	1 ^	В			'	U)	0°	/5°	30°	45°	0°	0°	15°	<i>30</i> °	45°	0°	/5°	<i>30</i> °	45°	0°	15°	30°	45°			ripe
12"	18"	1.3	2.6	3.9	5.2	1'-8"	1'-2"	3'-9"	1'-10"	1'-2"	0'-3"	2'-10"	2'-10"	2'-11"	3'-3"	4'-0"	1.09	1.45	1.46	1.51	1.60	1.80	1.82	1.91	2.09	2.16	2.20	2.33	2.60	12"	18"	15"
14"	23"	1.8	3.6	5 . 4	7.2	1'-10"	/'-3"	4' 원 "	/'-//"	/'-3"	8½"	3'-5"	3'-5"	3'-6"	3'-//"	4'-10"	1.36	1.82	1.84	1.89	2.01	2.29	2.32	2.43	2.68	2.75	2.80	2.97	3.33	14"	23"	18"
19"	30"	3.3	6.6	9.9	13.2	2'-3"	1'-4"	5'-12"	2'-0"	1'-4"	1'-72"	4'-2"	4'-2"	4'-4"	4'-10"	5'-//"	1.89	2.55	2.57	2.65	2.82	3.22	3.27	3.43	3.77	3.88	3.95	4.19	4.70	19"	30"	24"
24"	38"	5.1	10.2	/5.3	20.4	2'-8"	/'-5"	6'-3"	2'-1"	/'-5"	2'-9"	5'-2"	5'-2"	5'-4"	6'-0"	7'-4"	2.64	3.55	3.58	3.69	3.93	4.48	4.54	4.77	5.24	5.39	5.49	5.82	6.53	24"	38"	30"
29"	45"	7.4	14.8	22.2	29.6	3'-/"	1'-6"	7'-0"	2'-2"	1'-6"	3'-6"	6'-0"	6'-0"	6'-3"	6'-11"	8'-6"	3.32	4.48	4.52	4.66	4.96	5.64	5.72	6.00	6.60	6.80	6.92	7.34	8.24	<i>2</i> 9"	<i>4</i> 5"	36"
34"	53"	10.2	20.4	30.6	40.8	3'-6"	/'-7"	7'-112"	2'-3"	/'-7"	4' -5½"	7'-/"	7'-/"	7'-4"	8'-2"	10'-0"	4.24	5.76	5.8/	6.00	6.39	7.29	7.40	7.76	8.55	8.81	8.97	9.52	10.70	34"	53"	42"
38"	60"	12.9	25.8	38.7	51.6	3'-10"	1'-8"	8'-9"	2'-4"	1'-8"	5'-3"	7'-//"	7'-//"	8'-2"	9'-2"	//'-2"	5 . 22	7.16	7.23	7.46	7.96	9.10	9.24	9.70	10.71	11.05	// . 25	11.95	13.46	38"	60"	48"
43"	68"	16.6	33.2	49.8	66.4	4'-3"	1'-10"	9' -8½"	2'-6"	1'-10"	6'-2½"	8'-10"	8'-10"	9'-2"	10'-2"	12'-6"	6.63	9.01	9.09	9.38	10.00	11.39	11.56	12.13	13.36	13.77	14.02	14.88	16.73	43"	68"	54"
48"	76"	20.5	41.0	61.5	82.0	4'-8"	2'-1"	10'-8"	2'-9"	2'-0"	7'-2"	9'-9"	9'-9"	10'-1"	11'-3"	13'-9"	8.66	11.74	// . 85	12.22	13.02	14.82	15.04	15.77	17.37	17.91	18.23	19.34	21.74	48"	76"	60"
53"	83"	24.8	49.6	74.4	99.2	5'-1"	2'-6"	//'-7"	3'-2"	2'-6"	8'-1"	10' -7"	10'-7"	10'-11"	12'-3"	15'-0"	12.50	16.98	16.98	17.67	18.83	21.47	21.78	22.86	25.18	<i>25.9</i> 7	26.44	28.06	31.55	53"	83"	66"
58"	91"	29.5	59.0	88.5	118.0	5'-6"	2'-10"	12' -6½"	3'-6"	2'-10"	9'-0½"	//'-4"	//'-4"	11'-9"	13'-1"	16'-0"	16.46	22.26	22.46	23.16	24.66	28.05	28.46	29.85	<i>32.8</i> 5	<i>33.8</i> 5	34.46	36.55	41.05	58"	9/"	72"

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

STRAIGHT CONCRETE ENDWALLS SINGLE AND MULTIPLE PIPE

	Names	Dates	Approve	· 1 / /	V P			
Designed By	HAB/EGR	73/83	State Drainage Engineer					
Drawn By	RWR/HSD	83	Revision	Sheet No.	Index No.			
Checked By	JBW/JVG	83	00	2 of 2	250			



OPTIONAL ENTRANCE FOR CONCRETE PIPE

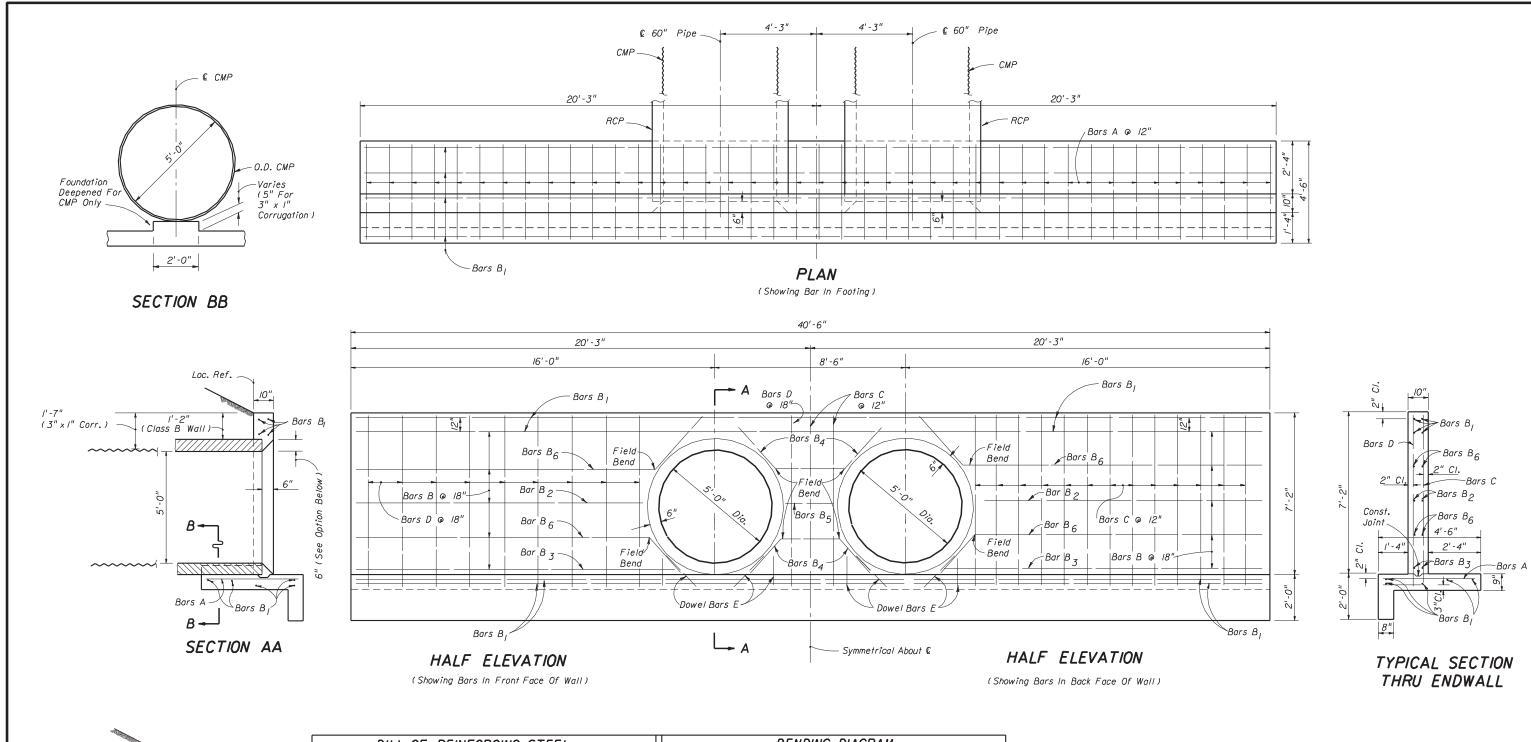
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- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II except concrete meeting the requirements of ASTM C 478 (4000 PSI) may be used in lieu of Class II concrete in precast units manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Concrete. Class II (Endwalls), CY and Reinforcing Steel (Roadway), LB.

STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 60" PIPE

	Names	Dates	Approve	d By	V D
Designed By				// لما لكر State Drainage E	(chemoso) Engineer
Drawn By	TWJ	11/49	Revision	Sheet No.	Index No.
Checked By	WHM	11/49	00	1 of 2	251





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OPTIONAL ENTRANCE FOR CONCRETE PIPE

BILL OF REINFORCING STEEL										
MARK	SIZE	NO. REO'D	LENGTH	LOCATION	BENDING					
Α	#4	41	4'-2"	Footing	Straight					
Вј	#4	9	40'-2"	Footing & Wall	Straight					
B ₂	#4	4	12'-6"	Wall	Straight					
В3	#4	4	13'-9"	Wall	Straight					
B4	#4	4	6'-0"	Wall	Field Bend					
B ₅	#4	2	2'-2"	Wa/I	Straight					
В6	#4	8	15'-0"	Wall	Field Bend					
С	#4	29	9'-4"	Footing & Wall	Bend					
D	#4	20	7'-6"	Footing & Wall	Straight					
Ε	#4	16	1'-8"	Footing & Wall	Straight					

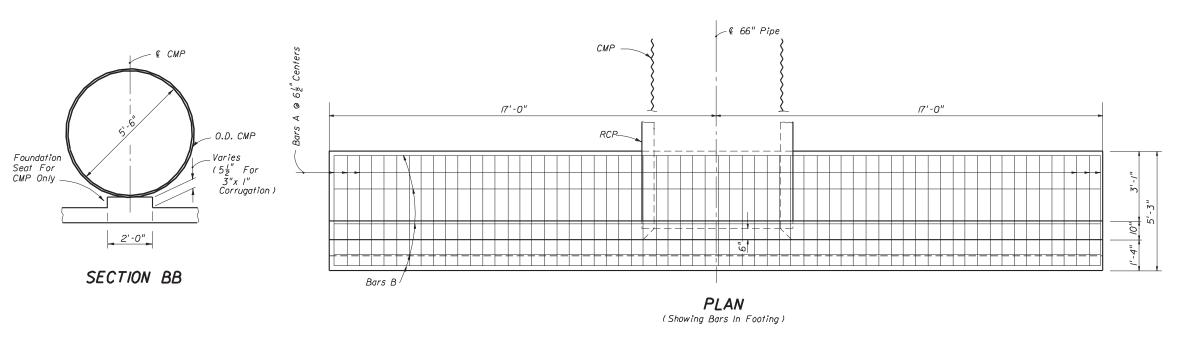
	BENDING DIAGRAM									
	"O!-,									
4	BAR C									
d	NOTE: All bar dimensions are out to out									
	ESTIMATED QUANTITIES									
d	ITEM	UNIT	RCP	CMP						
4	Concrete Class II	Cu. Yd.	13.7	13.8						
+	Reinforcing Steel	Lb.	824	824						

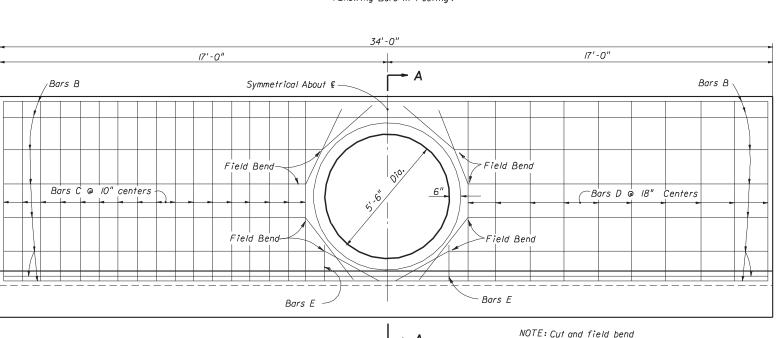
NOTE: See Sheet Lof 2 For General Notes.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

STRAIGHT CONCRETE ENDWALLS
SINGLE AND DOUBLE 60" PIPE

	Names	Dates	Approve	a By I A W	' p					
Designed By			State Drainage Engineer							
Drawn By	TWJ	11/49	Revision	Sheet No.	Index No.					
Checked By	WHM	11/49	00	2 of 2	251					





GENERAL NOTES

HALF ELEVATION

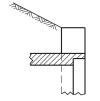
(Showing Bars In Back Face Of Wall)

HALF ELEVATION

Bars B as shown

(Showing Bars In Front Face Of Wall)

TYPICAL SECTION



Loc. Ref._

SECTION AA

∍Bars B

l'-2" (Class B Wall)

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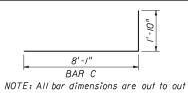
OPTIONAL ENTRANCE FOR CONCRETE PIPE

- I. Straight concrete endwalls are intended for use outside the clear zone.
- 2. Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the details on this index, design specifications AASHTO 1989. Precast construction which adheres to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index No. 201 for opening and grouting
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II except concrete meeting the requirements of ASTM C 478 (4000 psi) may be used in lieu of Class II concrete in precast units manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.

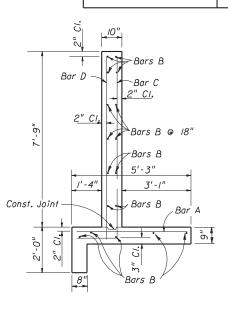
- 5. Chamfer: All exposed edges and corners to be chamfered $\frac{3}{4}$ unless otherwise shown.
- 6. Metal pipe shall be bituminous coated on all surfaces in contact with concrete and 12" beyond the boundary of contact. Any suitable bituminous material may be field applied.
- 7. Sodding shall be in accordance with Index No. 28I and paid for under the contract unit price for Sodding, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Concrete Class II (Endwalls), CY and Reinforcing Steel (Roadway), LB.

MARK SIZE NO. REO'D LENGTH LOCATION BENDING 4'-//" Footing Straight В 4 33'-8" Footing & Wall Straight С 5 34 9'-11" Wall Bend 4 D 20 8'-1" Wall Straight 4 1'-8" Wall Straight BENDING DIAGRAM

BILL OF REINFORCING STEEL



ESTIMATED QUANTITIES UNIT CMP Concrete Class II 13.3 Cu. Yd. 13.2 Reinforcing Steel Lb. 1170 1170



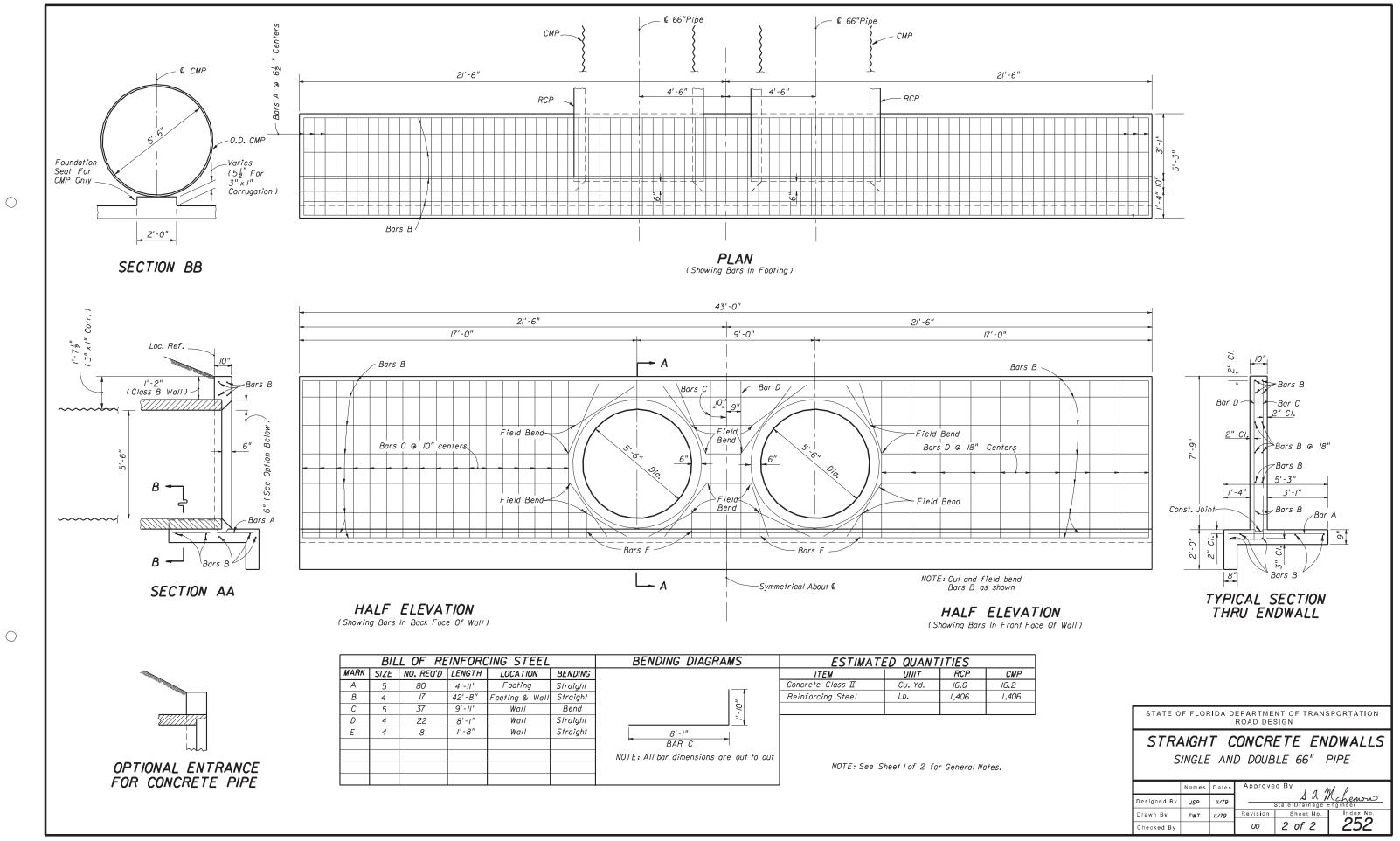
THRU ENDWALL

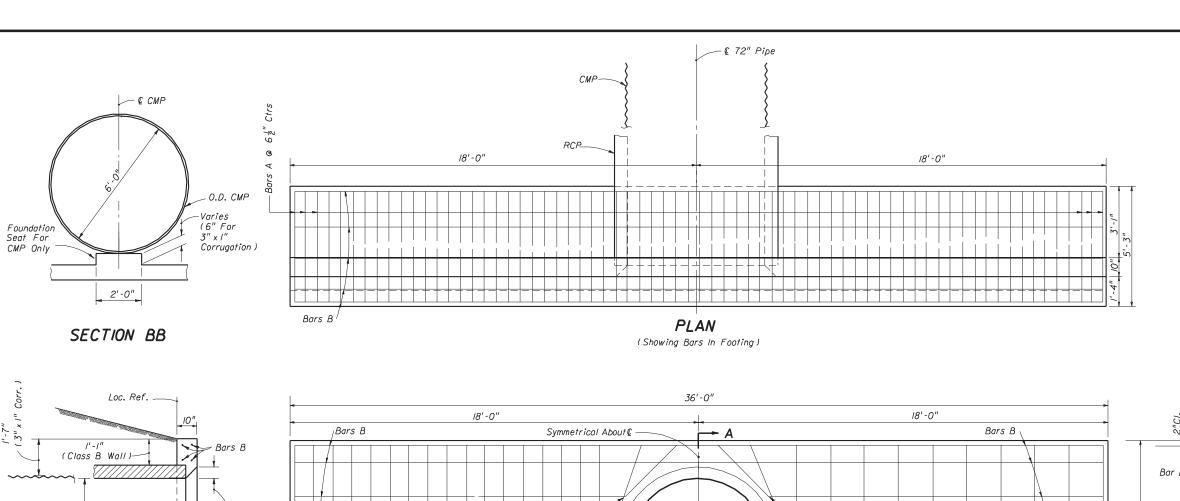
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

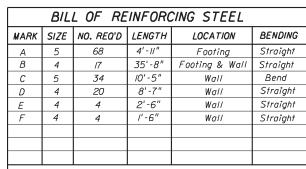
STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 66" PIPE

	Names	Dates	Approve	d By	10
esigned By	JLW	03/54		A N // State Drainage	(chemorio
rawn By			Revision	Sheet No.	Index No.
Checked By	RCB	03/54	00	1 of 2	252

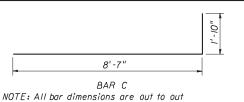
\$\$\$\$\$\$SYT/ME\$\$\$\$\$





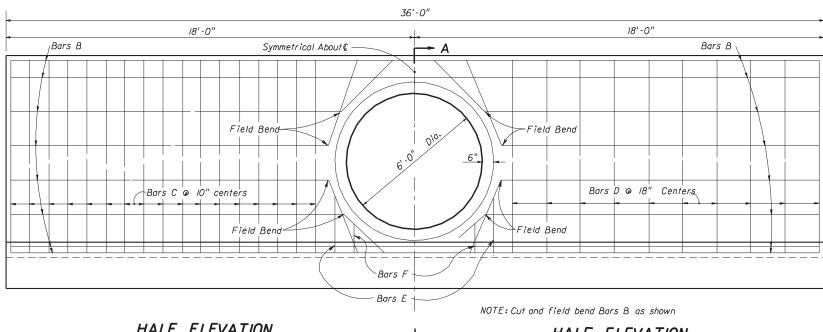


BENDING DIAGRAM



ESTIMATED QUANTITIES

ITEM	UNIT	RCP	CMP						
Concrete Class II	Cu. Yd.	14.4	14.5						
Reinforcing Steel	Lb.	1249	1249						



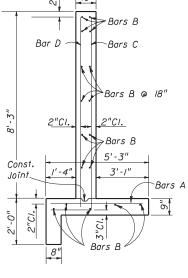
HALF ELEVATION

(Showing Bars In Back Face Of Wall)

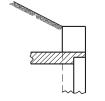
GENERAL NOTES

HALF ELEVATION

(Showing Bars In Front Face Of Wall)



TYPICAL SECTION THRU ENDWALL



SECTION AA

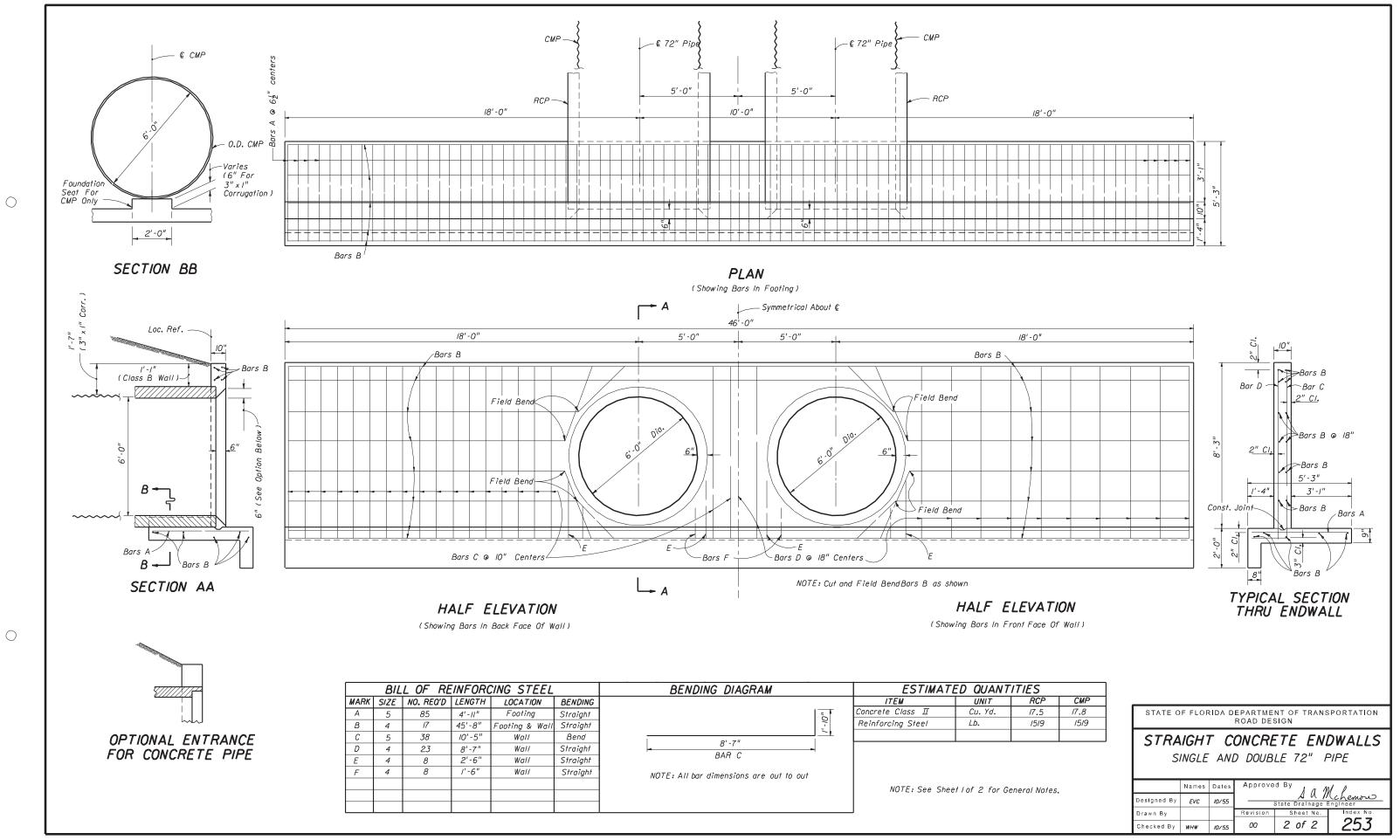
OPTIONAL ENTRANCE FOR CONCRETE PIPE

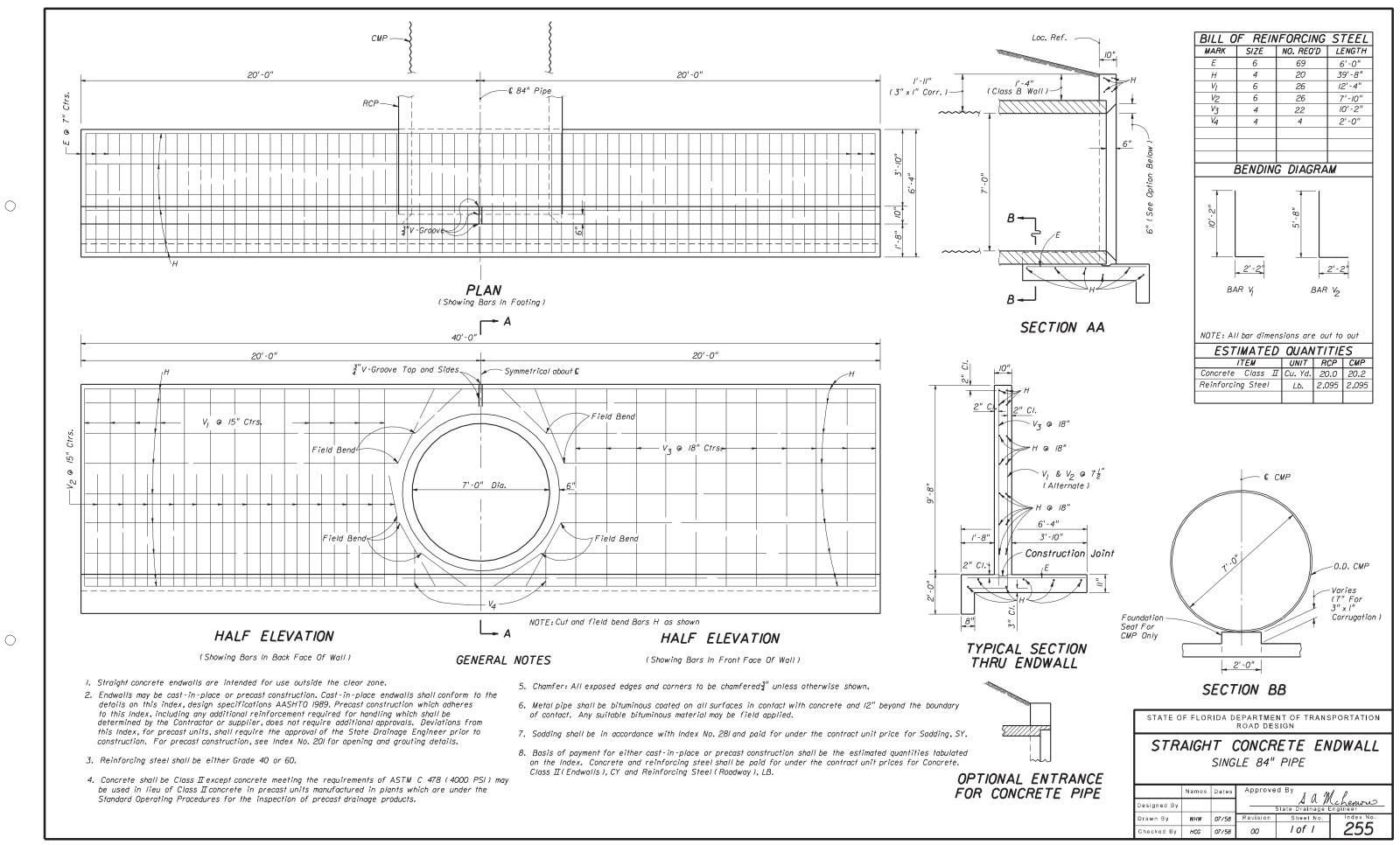
- I. Straight concrete endwalls are intended for use outside the clear zone.
- Endwalls may be cast-in-place or precast construction. Cast-in-place endwalls shall conform to the
 details on this index, design specifications AASHTO 1989. Precast construction which adheres
 to this Index, including any additional reinforcement required for handling which shall be determined by the Contractor or supplier, does not require additional approvals. Deviations from this Index, for precast units, shall require the approval of the State Drainage Engineer prior to construction. For precast construction, see Index No. 201 for opening and grouting details.
- 3. Reinforcing steel shall be either Grade 40 or 60.
- 4. Concrete shall be Class II except concrete meeting the requirements of ASTM C 478 (4000 PSI) may be used in lieu of Class II concrete in precast units manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.
- 5. Chamfer: All exposed edges and corners to be chamfered $\frac{3}{4}$ unless otherwise shown.
- 6. Metal pipe shall be bituminous coated on all surfaces in contact with concrete and I2" beyond the boundary of contact. Any suitable bituminous material may be field applied.
- 7. Sodding shall be in accordance with Index No. 28I and paid for under the contract unit price for Sodding, SY.
- 8. Basis of payment for either cast-in-place or precast construction shall be the estimated quantities tabulated on the Index. Concrete and reinforcing steel shall be paid for under the contract unit prices for Concrete. Class II (Endwalls), CY and Reinforcing Steel (Roadway), LB.

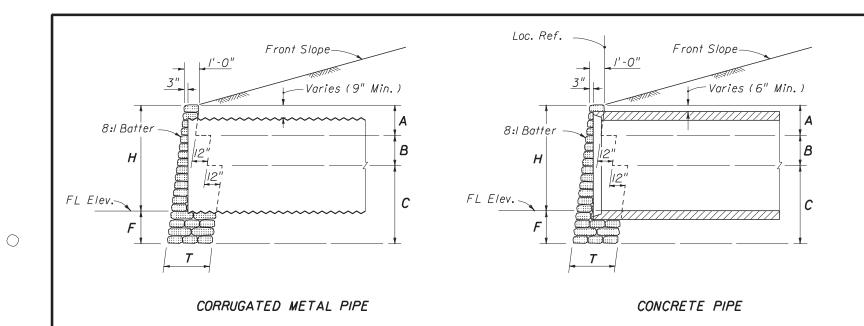
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

STRAIGHT CONCRETE ENDWALLS SINGLE AND DOUBLE 72" PIPE

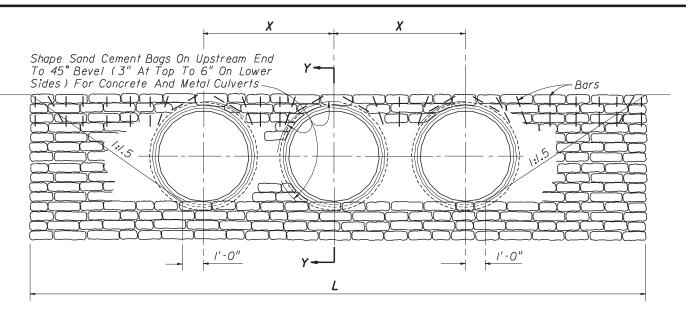
	Names	Dates	Approve	1/1/1/2	1 1		
Designed By	EVC	10/55	State Drainage Engineer				
Drawn By			Revision	Sheet No.	Index No.		
Checked By	WHW	10/55	00	1 of 2	253		







SECTION YY



Note: (1) For concrete and corrugated metal pipes. Concrete pipe shown.

- (2) The top row of riprap bags shall be secured by pinning, using #4 reinforcing bars
 - 18 inches in length, as follows:

 (a) The end bags shall be secured using two bars per bag, one vertical and one diagonal as shown.

 (b) The next to last bag on each end shall be secured with two bars vertically.
 - (c) Bags located over the pipe shall be secured by a bar which is driven diagonally except that for concrete pipe two bars shall be used for single bags above the pipe.
 - (d) Intermediate bags shall be secured with a single bar.

Bars shall be driven to one inch below the surface of the bag.

The cost of furnishing and installing the bars shall be included in the cost of the riprap.

FRONT ELEVATION

				TABLE	E OF	DIM	<i>MENSI</i>	DNS AN	ID	QUA	NTITIES	FC)R	ONE E	NDWA	<u> </u>			
SIZE								ONE PIPE	CULVE	RTS	TWO PIPE	CULVE	RTS	THREE PIPE	CULVE	ERTS	FOUR PIPE	CULVE	ERTS
OF_	<i>H</i>	T	Α	В	С	F	X	,	RIPRA	AP CY	,	RIPRA	P CY	,	RIPR	AP CY	,	RIPRA	AP CY
PIPE									CP	CMP	L	CP	СМР	L	CP	CMP		CP	CMP
18"	2'-3"	<i>1'-0"</i>	4'-0"	0'-0"	0'-0"	1'-9"	2'-10"	8'-9"	1.2	1.2	//'-7"	1.5	1.6	14' - 5"	1.8	1.9	17'-3"	2.1	2.3
24"	2'-9"	2'-0"	2'-0"	2'-6"	0'-0"	1'-9"	3'-5"	10'-3"	2.4	2.5	13'-8"	3.0	3.2	17'-1"	3.7	4.0	20'-6"	4.3	4.7
30"	3'-4"	2'-0"	2'-0"	3'-2"	0'-0"	1'-10"	4'-3"	12'-0"	3.3	3.4	<i>16' - 3"</i>	4.2	4. 5	20'-6"	5./	5.5	24'-9"	6.0	6.5
36"	3'-10"	2'-0"	2'-0"	3'-8"	0'-0"	1'-10"	5'-1"	13'-6"	4.0	4.2	<i>18'</i> -7"	5.2	5.7	23' -8"	6.3	6.9	28'-9"	7.4	8,2
42"	4'-5"	3'-0"	2'-0"	2'-0"	2'-4"	/'-//"	6'-0"	15' - 3"	6.4	6.7	21'-3"	8.3	8.9	27'-3"	10.2	11.2	33'-3"	12.3	13.4
48"	4'-//"	3'-0"	2'-0"	2'-0"	2'-10"	/'-//"	6'-9"	<i>16' -9"</i>	7.7	8./	23'-6"	10.0	10.8	30' - 3"	12.3	13.5	37'-0"	14.5	16.2
54"	5'-6"	3'-0"	2'-0"	2'-0"	3'-6"	2'-0"	7'-8"	18' -6"	9.5	10.1	26' - 2"	12.4	13.5	33' - 10"	15.3	17.0	41'-6"	18.2	20.4
60"	6'-0"	3'-0"	2'-0"	2'-0"	4'-0"	2'-0"	8'-6"	20'-0"	11.0	11.7	28' - 6"	14.4	15.8	37'-0"	17.8	19.8	45'-6"	21.1	23.8
66"	6'-7"	3'-0"	2'-0"	2'-0"	4'-8"	2'-1"	9'-3"	21'-9"	13.2	14.1	31'-0"	17.2	18.9	40' - 3"	21.2	23.7	49'-6"	25./	28.5
72"	7'-1"	3'-0"	2'-0"	2'-0"	5'-2"	2'-1"	10'-0"	23'-3"	15.0	16.0	33'-3"	19.4	21.4	43' - 3"	23.9	26.8	53'-3"	28.3	32.3
78"	7'-8"	3'-0"	2'-0"	2'-0"	5'-10"	2'-2"	10'-9"	25'-0"	17.5	18.7	35'-9"	22.6	25.0	46'-6"	27.8	31.3	57' - 3"	32.9	37.6
84"	8'-2"	3'-0"	2'-0"	2'-0"	6'-4"	2'-2"	11'-8"	26' -6"	19.5	20.9	38'-2"	25.3	28.1	49' - 10"	3/./	35.2	61' -6"	36.9	42.4

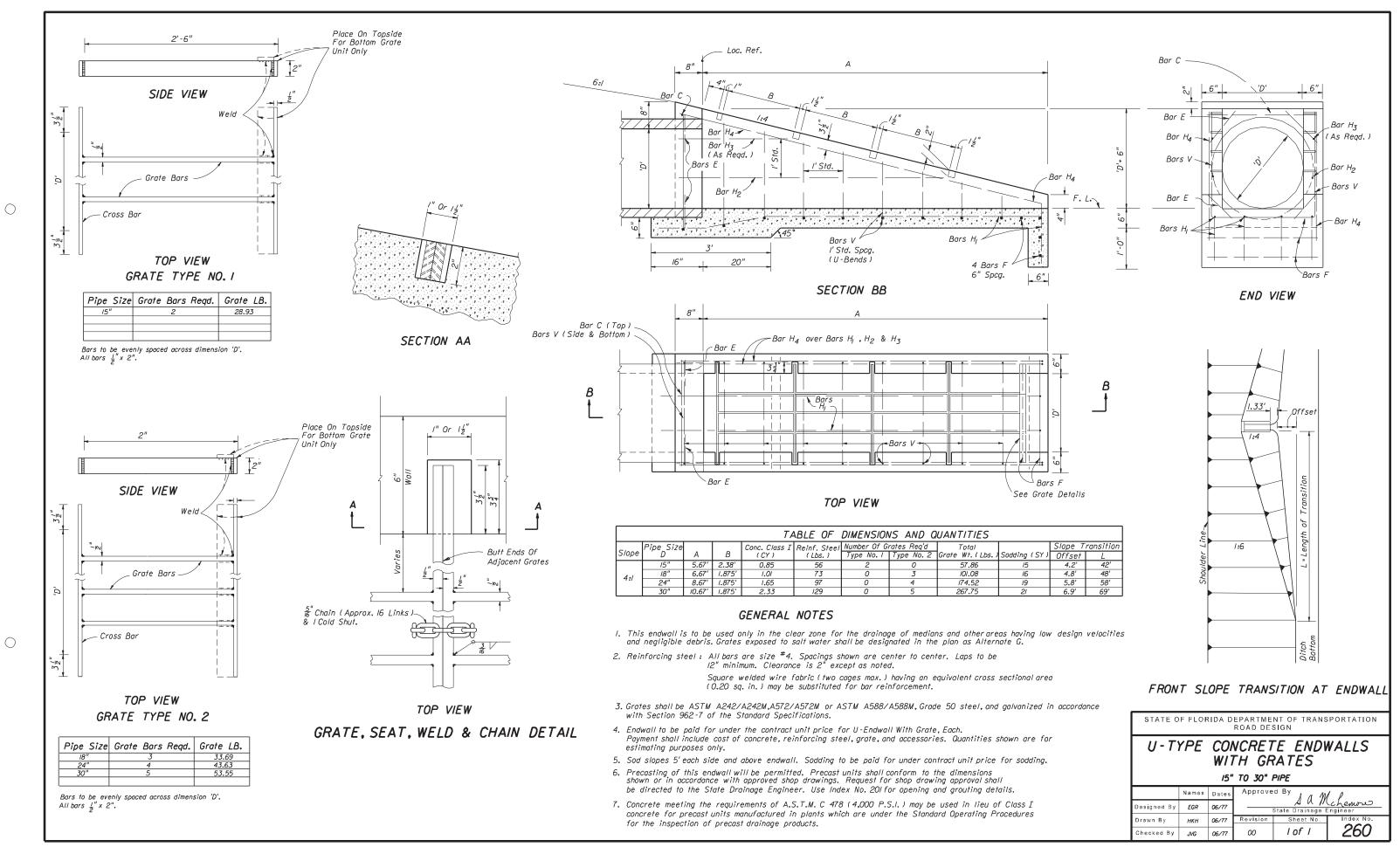
GENERAL NOTES

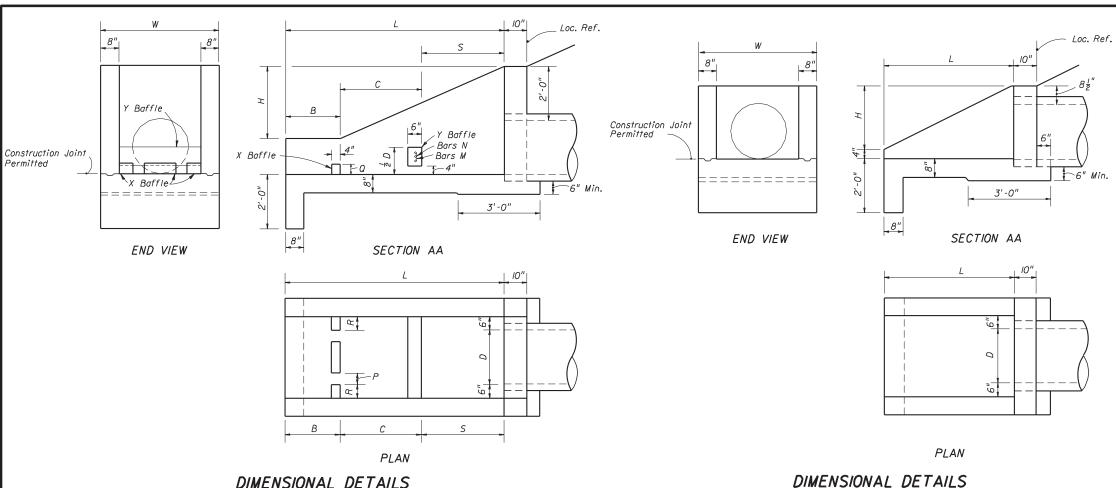
I. Straight sand-cement endwalls are intended for use outside the clear zone.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

STRAIGHT SAND-CEMENT **ENDWALLS**

	Names	Dates	Approve	d By I a Wa	chemous
Designed By				ا// کا لکر State Drainage E	Chemous Engineer
Drawn By	JBW	07/88	Revision	Sheet No.	Index No.
Checked By	JVG/EGR	08/88	00	l of l	258

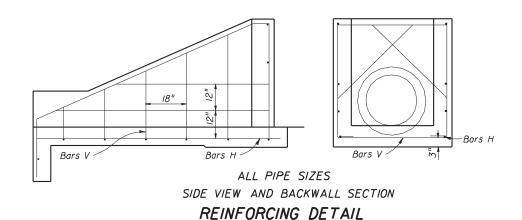




DIMENSIONAL DETAILS

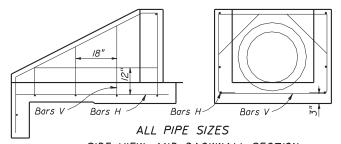
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DIMENSIONS AND QUANTITIES FOR ONE U-ENDWALL Baffle Reinf. Steel | Concrete Pipe Size X Baffle Steel Area Class I Lbs. Sq. Ft. Bar M Cu. Yd. 72 5'-9"|2'-3; 3'-7"| 2'-3" | 1'-3" 4" 2 #4 1#4 1.61 5" 3 #4 1.89 | 1.77 | 6'-6" | 2'-5" | 3'-10" | 2'-6" | 1'-6" | 2'-6" | 2 #4 86 6" 24" 3.14 8'-0" 2'-8" 4'-4" 3'-0" 2'-0" 3'-0" 3 #4 2.52 108 4 #4 30" 4.91 9'-6" 2'-11" 4'-10" 3'-6" 2'-6" 3'-6" 4 #4 3.34 131

WITH BAFFLES



SIDE VIEW AND BACKWALL SECTION REINFORCING DETAIL

DIMENS	IONS AN	ID QU	ANTIT	IES F	OR ONE	U-ENDWALL
Pipe	Size				Concrete	Reinf.
D	Area Sq. Ft.	L	Н	W	Class I Cu. Yd.	Steel Lbs.
/5"	1.23	3'-3"	1'- 71"	3'-7"	0.89	39
18"	1.77	3'-9"	/' - 10 ½"	3'-10"	1.05	43
24"	3.14	4'-9"	2' - 4 1/2"	4'-4"	1.40	55
30"	4.9/	5'-9"	2' - 10 ½"	4'-10"	1.88	64

WITHOUT BAFFLES

ENDWALLS FOR 2: 1 SLOPES

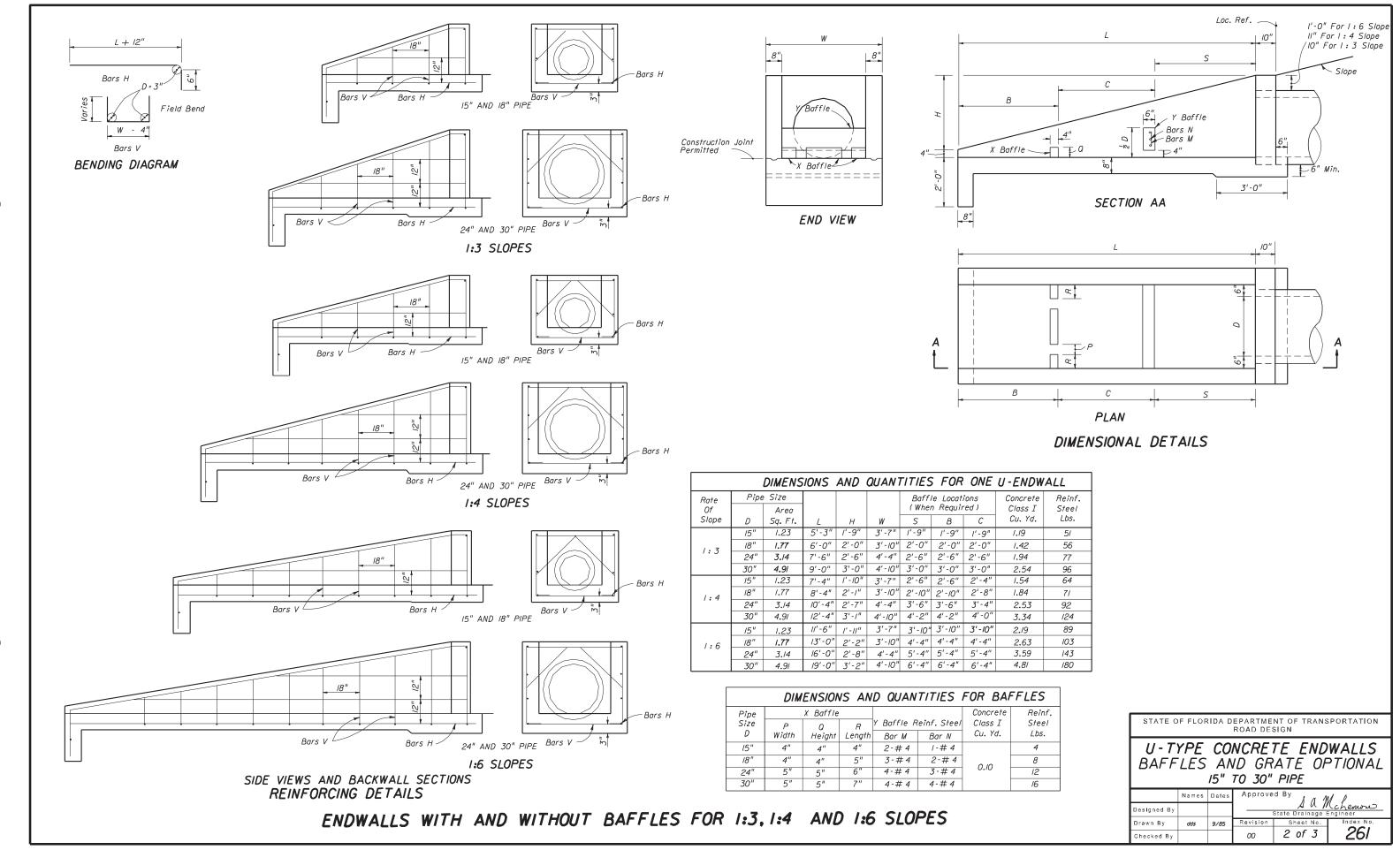
GENERAL NOTES

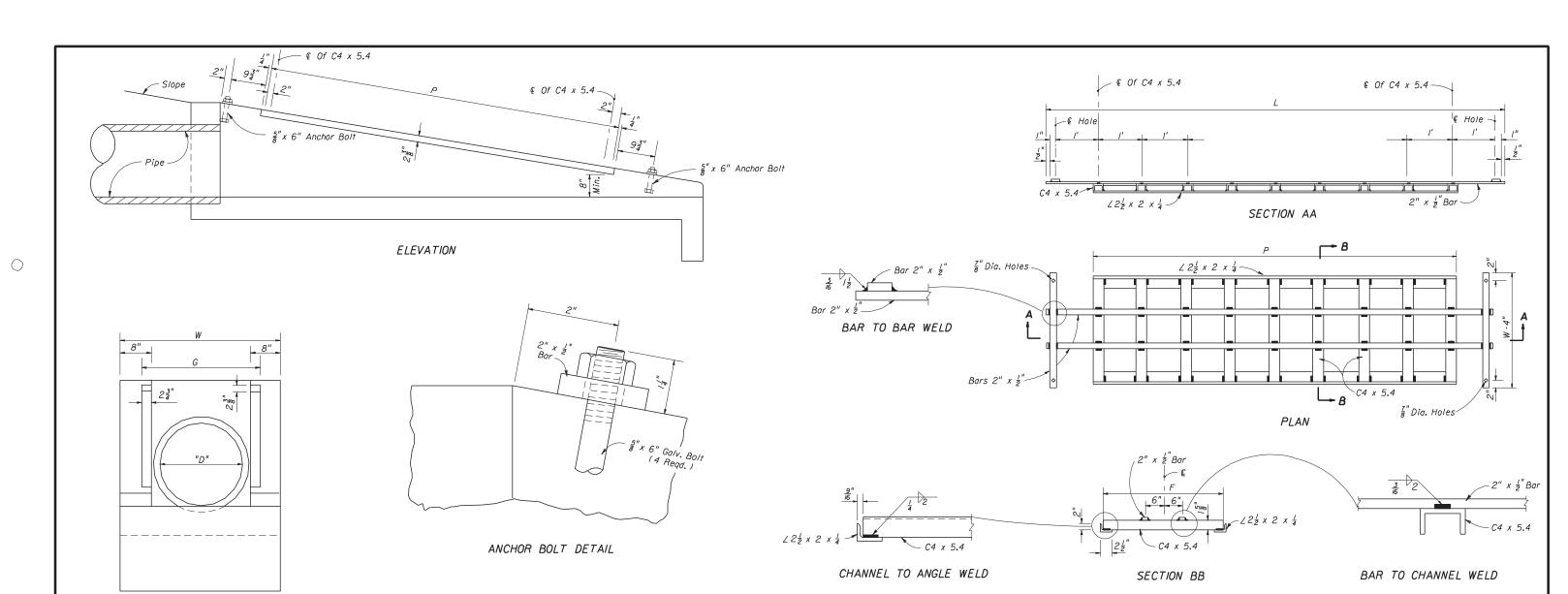
- I. Baffles to be constructed only when called for in plans.
- 2. When steel grating is required on endwall see Sheet 3 of 3 for details.
- 3. All reinforcing No. 4 bars with 2" clearance except as noted.
- 4. All angles, channels and bars shall be ASTM A242/A242M, A572/A572M or A588/A588M Grade 50 steel when designated Alternate G in the plans galvanized in accordance with Section 962-7 of the Standard Specifications.
- 5. Channel section C 3 x 6 may be substituted for C 4 x 5.4 channel.
- 6. Precasting of this endwall will be permitted. Precast units shall conform to the dimensions shown or in accordance with approved shop drawings. Request for shop drawing approval shall be directed to the State Drainage Engineer. Use Index No. 20I for opening and grouting details.
- 7. Concrete meeting the requirements of ASTM C-478 (4000 psi) may be used in lieu of Class I concrete in precast units manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.
- 8. Sodding shall be in accordance with Index No. 281, and paid for under the contract unit price for Sodding, SY.
- 9. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB. Cost of grates to be paid for under the contract unit price for Endwall Grate, LB, plan quantity. Cost of galvanized bolts and nuts to be included in the

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

U-TYPE CONCRETE ENDWALLS BAFFLES AND GRATE OPTIONAL 15" TO 30" PIPE

	Names	Dates	Approve	d By	10
Designed By			<u> </u>	ال لما لم State Drainage E	(chemosis)
Drawn By	dds	09/85	Revision	Sheet No.	Index No.
Checked By			00	1 of 3	261





MOUNTING FOR STEEL GRATE

STEEL GRATING USE CRITERIA

- I. Grates to be used on pipe culvert endwalls located within the designated clear zone. Positive debris control shall be provided at all upgradient openings.

 Grates shall not be used unless one or more of the following conditions exist:
- A. Drainage area to culvert consists of median or infield areas or areas where debris and/or drift is negligible.
- B. Runoff to culvert is by sheet flow or in such ill defined channels that debris transport is not considered a major problem.
- C. Runoff to culvert is minor except on an infrequent basis (10 to 15 year frequency); for example a drainage basin in flat sandy terrain with normally low ground water table.

END VIEW

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- D. Areas where culvert blockage with resultant backwater would not seriously affect roadway embankment, traffic operation or upland property.
- 2. Steel grating to be used only where called for in plans.

		TABLE	OF DI	MENSION	IS AND	QUANTI	TIES FOR	R ONE (GRATE		
Rate Of	Size Pipe		2 Each B	ars @ 3.4	Lbs./L.F.	(X) Cho	innels @ 5.4	Lbs./L.F.		gles @ bs./L.F.	Total
Slope	" D "	G	L	W - 4"	Lbs.	(X)	F	Lbs.	P	Lbs.	Weight Lbs.
				_, _,	25	_	-7"		"		0.40
	15"	2' -8½"	9'-3"	3'-3"	85	8	2' -67"	///	7'-4"	53	249
1 : 6	18"	2' -11 ½"	10'-3"	3'-6"	94	9	2' - 9 7 "	137	8'-4"	62	292
	24"	3' -5½"	/3'-3"	4'-0"	117	12	3' - 3 7 "	215	//'-4"	82	414
	30"	3' -//2 ["]	16' - 3"	4'-6"	141	<i>1</i> 5	3' -9 7 "	310	14'-4"	104	555
	<i>15"</i>	2' -8½"	6'-3"	3'-3"	65	5	2' -67"	70	4'-4"	32	167
1:4	18"	2' - 11 ½"	7'-3"	3'-6"	73	6	2' - 97"	92	5'-4"	<i>3</i> 9	204
	24"	3' -5½"	9'-3"	4'-0"	90	8	3' -37"	144	7'-4"	53	287
	30"	3' -// <u>2</u> "	//'-3"	4'-6"	107	10	3' -9 7 "	206	9'-4"	68	381
	<i>15"</i>	2' -8½"	4'-3"	3'-3"	5/	3	2' -67"	42	2'-4"	17	110
/ : 3	18"	2' -11 ½"	5'-3"	3'-6"	60	4	2' - 9 7 "	6/	3'-4"	24	145
	24"	3' -5 <u>1</u> "	6'-3"	4'-0"	70	5	3' -3 7 "	90	4'-4"	31	191
	30"	3' -// =/"	8'-3"	4'-6"	87	7	3' -9 7 "	145	6'-4"	46	278

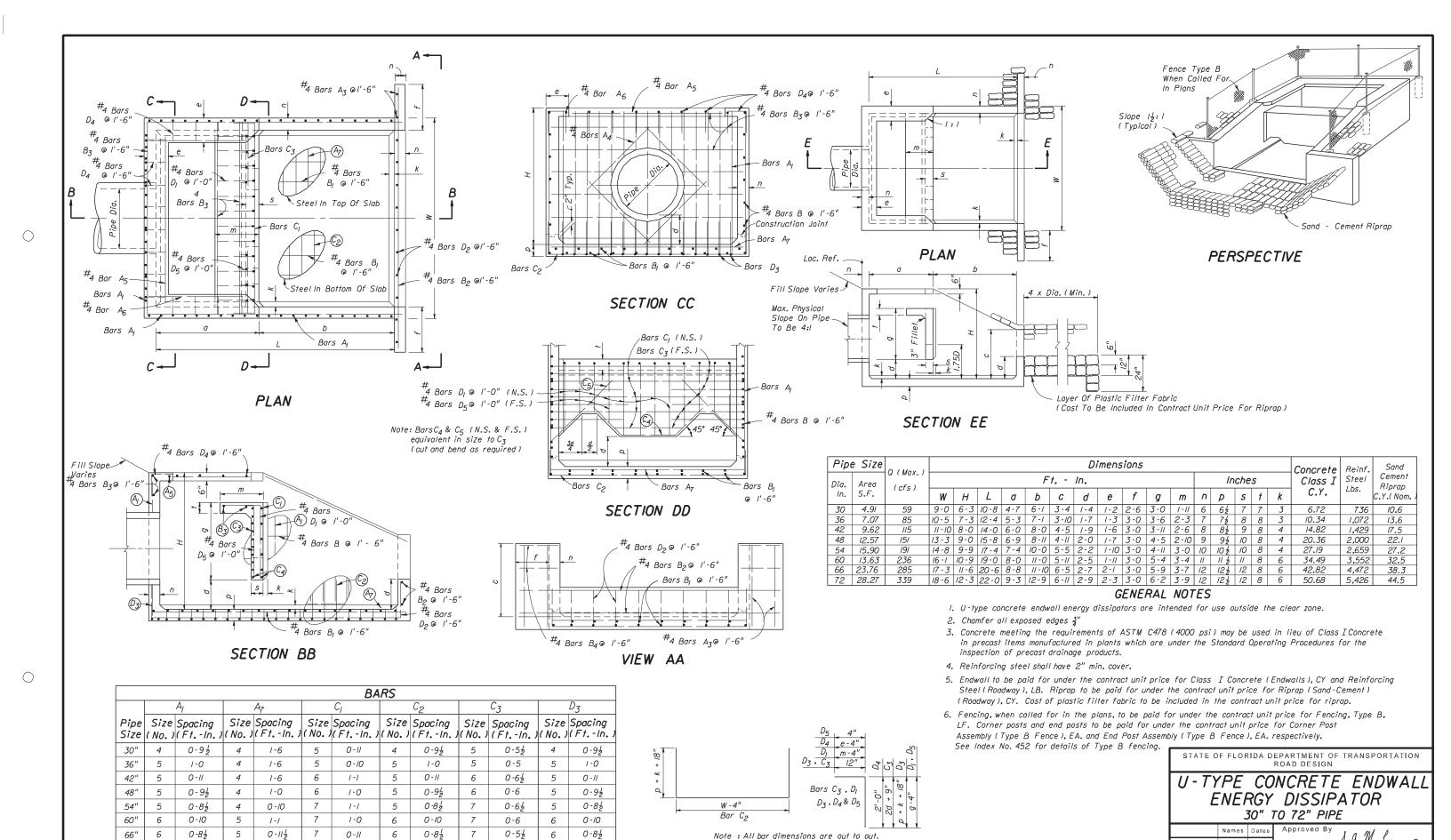
STEEL GRATE

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

U-TYPE CONCRETE ENDWALLS BAFFLES AND GRATE OPTIONAL

15" TO 30" PIPE

	Names	Dates	Approve	u by I A M	1 0
Designed By				// لما لمر State Drainage E	Chemoro Engineer
Drawn By	CDP	07/71	Revision	Index No.	
Checked By			00	3 of 3	261



BENDING DIAGRAM

0-5

0-7/2

6

0-75

6

0-10

72"

0-75

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0-10

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Designed By

Checked By JVG

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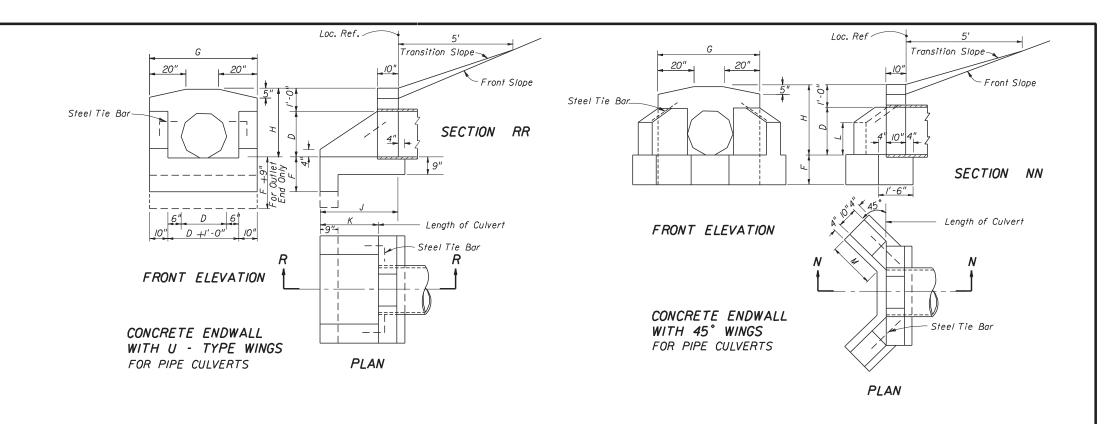


TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH U - TYPE WINGS

		L	DIMENS	IONS					QU.	ANT/T/	ES IN	ONE E	NDWALL
Open	ning		Wall		Foo	oting	To	tal Cu.	Yds. C	oncrete	e, Clas	s I	Stee!
D	Area	G	Н	К	F	,	Conc.	Pipe	C.M.	Pipe	C./.	Pipe	Tie Bars
	Sq.Ft.	G		^		J	Inlet Outlet		Inlet	Outlet	Inlet	Outlet	THE DUIS
12"	0.8	3'-8"	2'-0"	1'-0"	1'-3"	2'-2"	0.48	0.55	0.49	0.57	0.49	0.57	none
15"	1.2	3'-11"	2'-3"	1'-5"	1'-3"	2'-7"	0.59	0.67	0.62	0.70	0.61	0.70	none
18"	1.8	4'-2"	2'-6"	1'-9"		2'-11"	0.70	0.79	0.74	0.82	0.74	0.82	none
24"	3./	4'-8"	3'-0"	2'-6"	1'-6"	3'-8"	1.01	1.11	1.06	1.16	1.06	1.16	2 -3"0 x 2'-0"
30"	4.9	5'-2"		3'-3"	1'-6"		1.33	1.44	1.41	1.51	1.40	1.51	2 -3"0 x 2'-0"
36"	7.1	5'-8"	4'-0"	4'-0"	1'-9"	5'-2"	1.73	1.85	1.84	1.96	1.82	1.94	2 - 3"0 x 2'-6"
42"	9.6	6'-2"	4'-6"	4'-9"	2'-0"	5'-11"	2.19	2.32	2.32	2.45			2 -3"0 x 2'-6"
48"	12.6	6'-8"	5'-0"	5'-6"	2'-0"	6'-8"	2.64	2.78	2.81	2.95			2 -3"0 x 3'-0"

TABLE OF DIMENSIONS AND ESTIMATED QUANTITIES PIPE CULVERT ENDWALLS WITH 45° WINGS

		D	IMENSIC	DNS				QUANTITIE	ES IN ONE	ENDWALL		
Оре	ening		W	all		Footing	Co	ncrete, Cla	ss I			
D	Area	Н	G	,	М	F		Total Cu. Ya	s.	Steel Tie Bars		
	Sq.Ft.	П	G	L	l W		Conc. Pipe	C.M. Pipe	C.I. Pipe			
18"	1.8	2'-6"	3'-10"	1'-2"	/'-7"	1'-3"	0.74	0.77	0.77	none		
24"	3.1	3'-0"	4'-4"	l'-5"	2'-1"	1'-4"	1.01	1.06	1.06	2 - ¾"0 x 2'-0"		
30"	4.9	3'-6"	4'-10"	1'-9"	2'-5"	1'-6"	1.32	1.40	1.39	2 -3"0 x 2'-0"		
36"	7.1	4'-0"	5' <i>-4</i> "	2'-0"	2'-11"	1'-8"	1.72	1.83	1.82	2 - 3"0 x 3'-0"		
42"	9.6	4'-6"	5'-10"	2'-3"	3'-6"	2'-0"	2.34	2.47		2 -3"0 x 3'-0"		
48"	12.6	5'-0"	6'-4"	2'-6"	4'-0"	2'-0"	2.74	2.90		2 -3"0 x 3'-0"		
/5"	1.2	2'-3"	3'-7"	1'-0"	1'-3"	1'-3"	0.56	0.59	0.59	none		

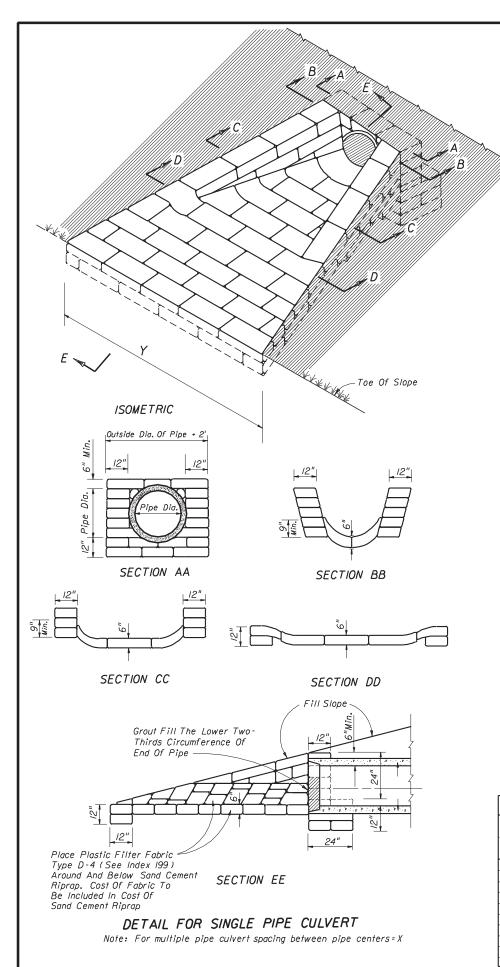
GENERAL NOTES

- I. Winged concrete endwalls are intended for use outside the clear zone.
- 2. Chamfer all exposed edges 3"
- 3. Concrete meeting the requirements of ASTM C-478 (4000 psi) may be used in lieu of Class I concrete in precast units manufactured in plants which are under the Standard Operating Procedures for the inspection of precast drainage products.
- 4. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY. Cost of steel tie bars to be included in the contract unit price for Class I Concrete.
- Sodding to be in accordance with Index No. 281, and paid for under the contract unit price for Sodding, Sy.

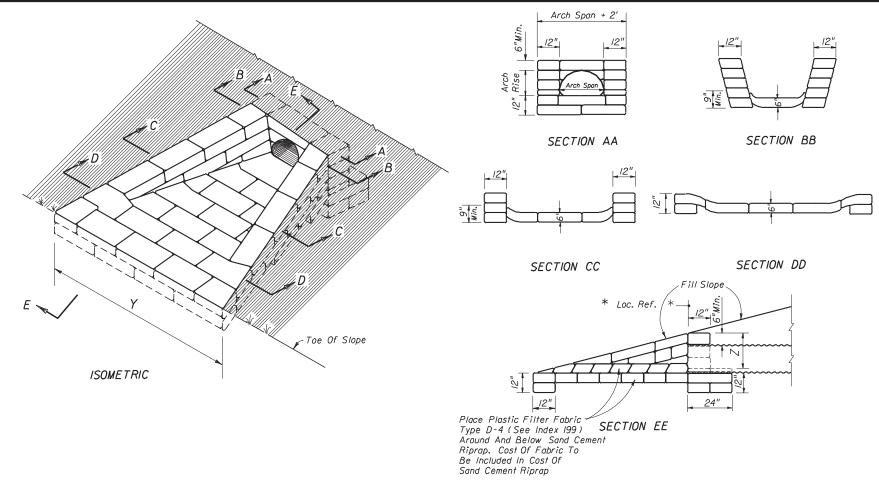
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

WINGED CONCRETE ENDWALLS SINGLE ROUND PIPE

	Names	Dates	Approve	d By	nd p			
Designed By			State Drainage Engineer					
Drawn By	TJK	12/31	Revision	Sheet No.	Index No.			
Checked By	GEF	03/32	00	I of I	266			



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DETAILS FOR SINGLE METAL PIPE ARCH CULVERTS

NOTE: For multiple metal pipe arch culvert spacing between arch centers = X

				DIM	ENSIC	NS A	ND QU	JANTI	TIES	FOR I	WETAL	PIPE	E ARC	H CU	LVERT	rs			
				Dime	ensions	5			Quan	tity of	Sand -	Cemen	t Ripro	ap in (Cu. Yds	. for O	ne En	dwall	
Span	Rise	Х		•	Υ		7	Fa	r 1:2	Slopes		F	or 1:4	Slopes	;	F	or 1:6	Slopes	
		^	I-Arch	2-Arch	3-Arch	4-Arch	4	I-Arch	2-Arch	3-Arch	4-Arch	I-Arch	2-Arch	3-Arch	4-Arch	I-Arch	2-Arch	3-Arch	4-Arch
17"	/3"	2'-6"	6'-6"	9'-0"	11'-6"	14'-0"	1'-7"	1.0	1.5	2.0	2.5	1.5	2.2	2.9	3.6				
21"	/5"	2'-10"	7'-6"	10'-4"	13'-2"	16'-0"	1'-9"	1.2	1.8	2.4	3.0	1.9	2.7	3.5	4.3				
28"	20"	3'-5"	9'-3"	12'-8"	16'-1"	19' -6"	2'-0"	1.7	2.5	3.3	4./	2.6	3.7	4.8	5.9				
35"	24"	4'-0"	//'-0"	15'-0"	19'-0"	23'-0"	2'-0"	2.2	3./	4.0	4.9	3.4	4.7	6.0	7.3				
42"	29"	4'-9"	12'-9"	17'-6"	22'-3"	27'-0"	2'-0"	2.9	4.1	5.3	6.5	4.5	6.1	7.7	9.3				
49"	33"	5'-6"	14'-6"	20'-0"	25'-6"	31'-0"	2'-0"	3.5	4.9	6.3	7.7	5.5	7.4	9.3	11,2				
57"	38"	6'-4"	16'-6"	22'-10"	29'-2"	35'-6"	2'-0"	4.4	6./	7.8	9.5	6.9	9.2	11.5	13.8				
64"	43"	7'-/"	18'-3"	25'-4"	32'-5"	39'-6"	2'-0"	5./	7.0	8.9	10.8	8./	10.7	13.3	15.9				
71"	47"	7'-10"	20'-0"	27'-10"	35'-8"	43'-6"	2'-0"	5.9	8.1	10.3	12.5	9.5	12.4	15.3	18.2				

GENERAL NOTES

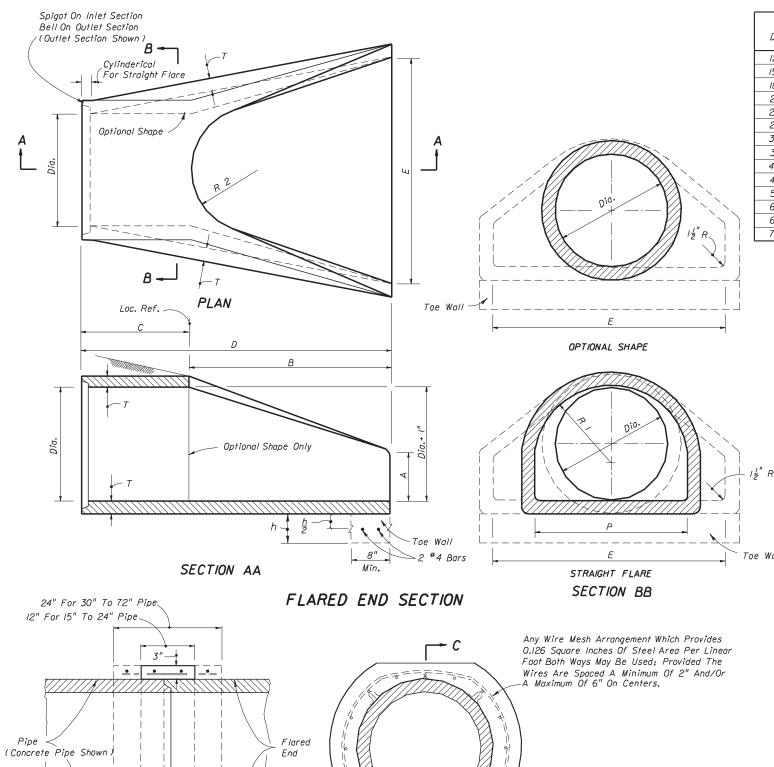
I. U-Type Sand-Cement Endwalls Are Intended For Use Outside The Clear Zone.

			DIML	ENSIO	VS AN	ID QUANTITIES FOR ROUND PIPE CULVERTS													
		Din	nension	s			Qua	ntity o	f Sand	Cemen	nt Ripro	p in C	u. Yds.	for On	e Endw	rall			
Pipe)	/			For 1:2				For I:4						Slopes		
Dia.	X I-Pipe 2-Pipes 3-Pipes 4-Pipe			4-Pipes	I-Pipe	2-Pipes	3-Pipes	4-Pipes	I-Pipe	2-Pipes	3-Pipes	4-Pipes	I-Pipe	2-Pipes	3-Pipes	4-Pipes			
/5"	2'-7"	7'-0"	9'-7"	12'-2"	14'-9"	1.2	1.6	2.1	2.6	1.7	2.4	3.0	3.6						
18"	2'-10"	8'-0"	10' -10"	13'-8"	16'-6"	1.4	2.0	2.6	3./	2.1	2.9	3.7	4.4						
24"	3'-5"	10'-0'	13'-5"		20'-3"	1.9	2.7	3. 5	4.3	2.9	4.0	5./	6.3						
30"	4'-3"	12'-0"			24'-9"	2 . 5	3. 6	4.8	5.9	3.8	5.4	7.0	8.6						
36"	5'-1"	14'-0"	19'-1"		29'-3"	3./	4.6	6.2	7.7	4.8	7.0	9.2	11.4						
42"	6'-0"	16'-0"	22'-0"		34'-0"	3.8	5.8	7.7	9.7	6.0	8.8	11.7	14.5						
48"	6'-9"	18'-0"	24' - 9"	31'-6"	38'-3"	4.5	7.0	9.4	11.8	7.2	10.8	14.3	17.9						
54"		20'-0"	27'-8"		43'-0"	5.3	8.3	11.3	14.2	8.5	12.9	17.3	21.7						
60"	8'-6"	22'-0"	30'-6"	39'-0"	47'-6"	6.2	9.7	13.3	16.9	10.0	15.3	20.6	<i>2</i> 5.9						

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION	
ROAD DESIGN	

U-TYPE SAND-CEMENT ENDWALLS

-		Names	Dates	Applove	111	W P
	Designed By	JEP	12/48		ال لکر State Drainage E	(chemoro
	Drawn By	HW	03/54	Revision	Sheet No.	Index No.
	Checked By	CDD	03/54	00	I of I	268



END VIEW

SECTION CC

REINFORCED CONCRETE JACKET DETAIL

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	DIA.	Т	REINF REINF. SO IN/LF	BELL Or SPIGOT	А	В	С	D	Ε	P	R I	R 2	FLAT	WEIGHT (LBS.)	h	TOE WALL CLASS I CONC (Misc.) CY
	12"	2"	0.07	/ <u>/</u> "	4"	2'-0"	4' - 0 <u>7</u> "	6'-0 <u>7</u> "	2'-0"	19 <u>15</u> "	10 8"	9"	3 <u>/</u> "	530	12"	.06
	<i>1</i> 5"	21/	0.07	2"	6"	2'-3"	3'-10"	6'-1"	2'-6"	24 <u>5</u> "	12 ½"	//"	3½"	740	12"	.07
	18"	2½"	0.07	2½"	9"	2'-3"	3'-10"	6'-1"	3'-0"	29"	15 ½"	12"	4"	990	<i>1</i> 5"	.//
	21"	23"	0.07	24"	9"	2'-11"	3'-2"	6'-1"	3'-6"	31 § ″	16 ½"	13"	4"	1280	<i>1</i> 5"	.12
	24"	3"	0.07	2 <u>1</u> "	9 <u>1</u> "	3'-7½"	2'-6"	6' - 1 <u>1</u> "	4'-0"	33 3 "	16 13"	14"	4½"	1520	18"	.17
	27"	34"	0.148	2 <u>1</u> "	10 ½"	4'-0"	2'- 1½"	6'-1 <u>2</u> "	4'-6"	36"	18 <u>9</u> "	14 ½"	4 <u>1</u> "	1930	18"	.19
	<i>30"</i>	3½″	0.148	3"	1'-0"	4'-6"	1'-73"	6'-/ 3 "	5'-0"	37"	18 ½"	<i>1</i> 5"	5"	2190	21"	.24
	36"	4"	0.148	3₺"	1'-3"	5'-3"	2' - 10 3"	8'-13"	6'-0"	47 <u>13</u> "	24 <u>5</u> "	20"	5½"	4100	21"	.29
	42"	41/2"	0.148	33/	1'-9"	5'-3"	2'-11"	8'-2"	6'-6"	53 7 "	27 ½"	22"	5½"	5380	24"	.36
	48"	5"	0.148	4 <u>1</u> "	2'-0"	6'-0"	2'-2"	8'-2"	7'-0"	56 ½"	28 ½"	22"	53"	6550	24"	.39
	54"	5½"	0.174	43"	2'-3"	5'-5"	2'-11"	8'-4"	7'-6"	65 <u>∤</u> "	33 / "	24"	6 ¼ "	8040	24"	.42
ĺ	60"	6"	0.174	5"	2'-6"	5'-0"	3'-3"	8'-3"	8'-0"	72 ½"	36 "	24"	63"	8750	24"	.44
	66"	6½"	0.174	5½"	2'-0"	6'-6"	1'-9"	8'-3"	8'-6"	72"	36 g'''	24"	7 4"	10630	24"	.47
	72"	7"	0.174	6"	2'-0"	6'-6"	1'-9"	8'-3"	9'-0"	77 <u>13</u> "	38 <u>15</u> "	24"	73"	12520	24"	.50

GENERAL NOTES

- I. Flared end sections shall conform to the requirements of ASTM C76 with the exception that dimensions and reinforcement shall be as prescribed in the table above. Circumferential reinforcement may consist of either one cage or two cages of steel. Compressive strength of concrete shall be 4000 psi. Shop drawings for flared end sections having dimensions other than above must be submitted for approval to the State Drainage Engineer.
- 2. Connections between the flared end section and the pipe culvert may be any of the following types unless otherwise shown on the plans.
 - a. Joints meeting the requirements of Section 941-1.5 of the Standard Specifications (O-Ring Gasket).

 Flared end section joint dimensions and tolerances shall be identical or compatible to those used in the pipe culvert joint. When pipe culvert and flared end section manufacturers are different, the compatibility of joint designs shall be certified to by the manufacturer of the flared end sections.
 - b. Joints sealed with preformed plastic gaskets.

 The gaskets shall meet the requirements of Section 942-2 of the Standard Specifications and the minimum sizes for gaskets shall be as that specified for equivalent sizes of elliptical pipe.
 - c. Reinforced concrete jackets, as detailed on this drawing.
 Cost of the reinforced concrete jacket to be included in the contract unit price for the flared end section.
 When non-coated corrugated metal pipe is called for in the plans, the pipe shall be bituminous coated in the
 jacketed area as specified on Index No. 280. Bituminous coating to be included in the contract unit price
 for the pipe culvert. Concrete jacket shall be as specified on index No. 280. Cost of concrete and reinforcement
 shall be included in the contract unit price for the pipe culvert.
- 3. Toe walls shall be constructed when shown on the plans or at locations designated by the Engineer. Toe walls are to be cast-in-place with Class I Concrete and paid for under the contract unit price for Class I Concrete (Miscellaneous), CY. Reinforcing steel to be included in cost of toe wall.
- 4. On skewed pipe culverts the flared end sections shall be placed in line with the pipe culvert. Side slopes shall be warped as required to fit the flared end sections.
- 5. Flared End Section to be paid for under the contract unit price for Flared End Section (Concrete), Each. Sodding shall be in accordance with Index No. 281, and paid for under the contract unit price for Sodding, SY.

DESIGN NOTES

I. Flared end sections are intended for use outside the clear zone on median drain and cross drain installation, except that flared end sections for pipe sizes I2" and I5" are permitted within the clear zone. When the slope intersection permits, I2" and I5" flared end sections may be located with the culvert opening as close as 8' beyond the outside edge of the shoulder.

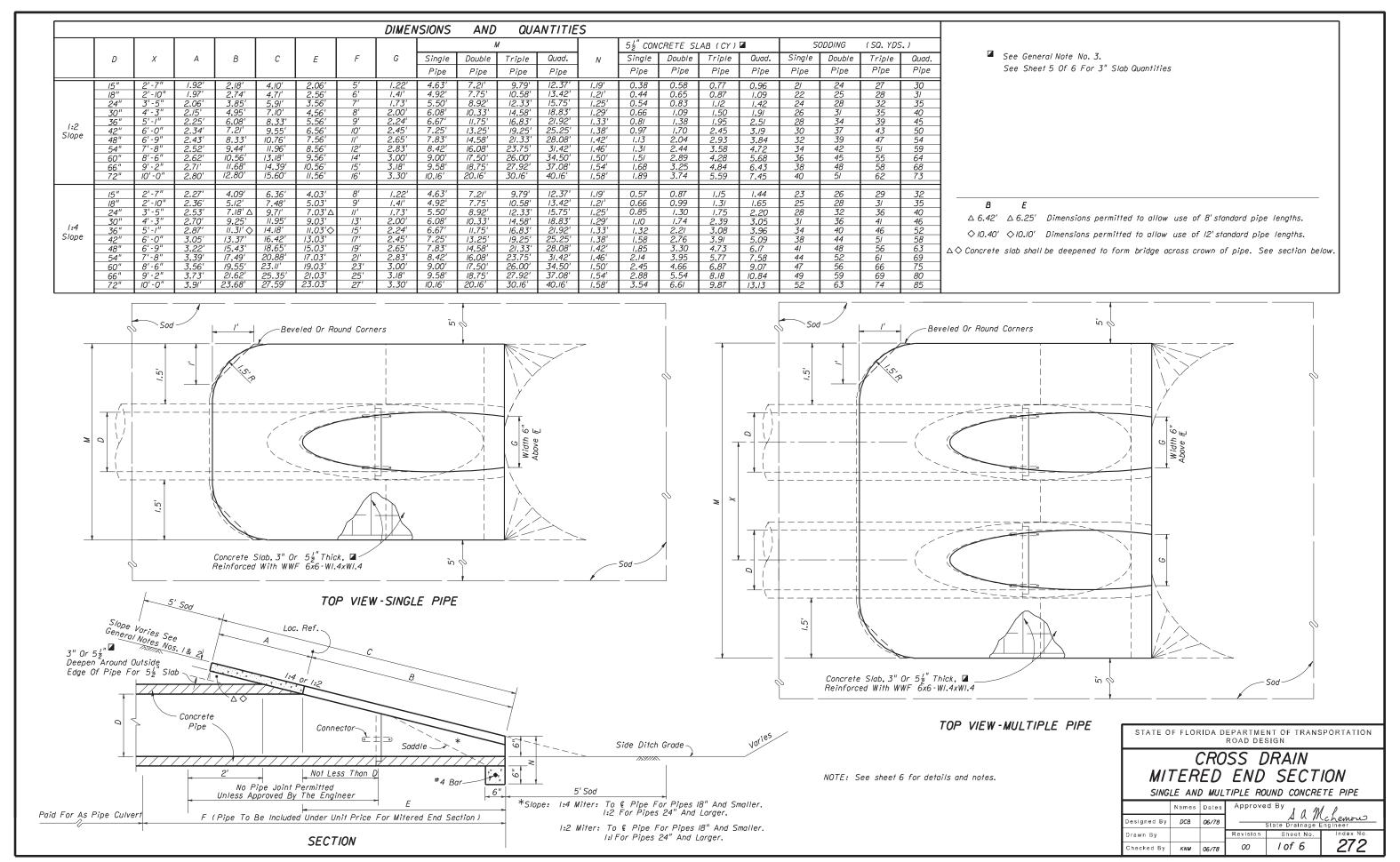
Flared end sections are not intended for side drain installations.

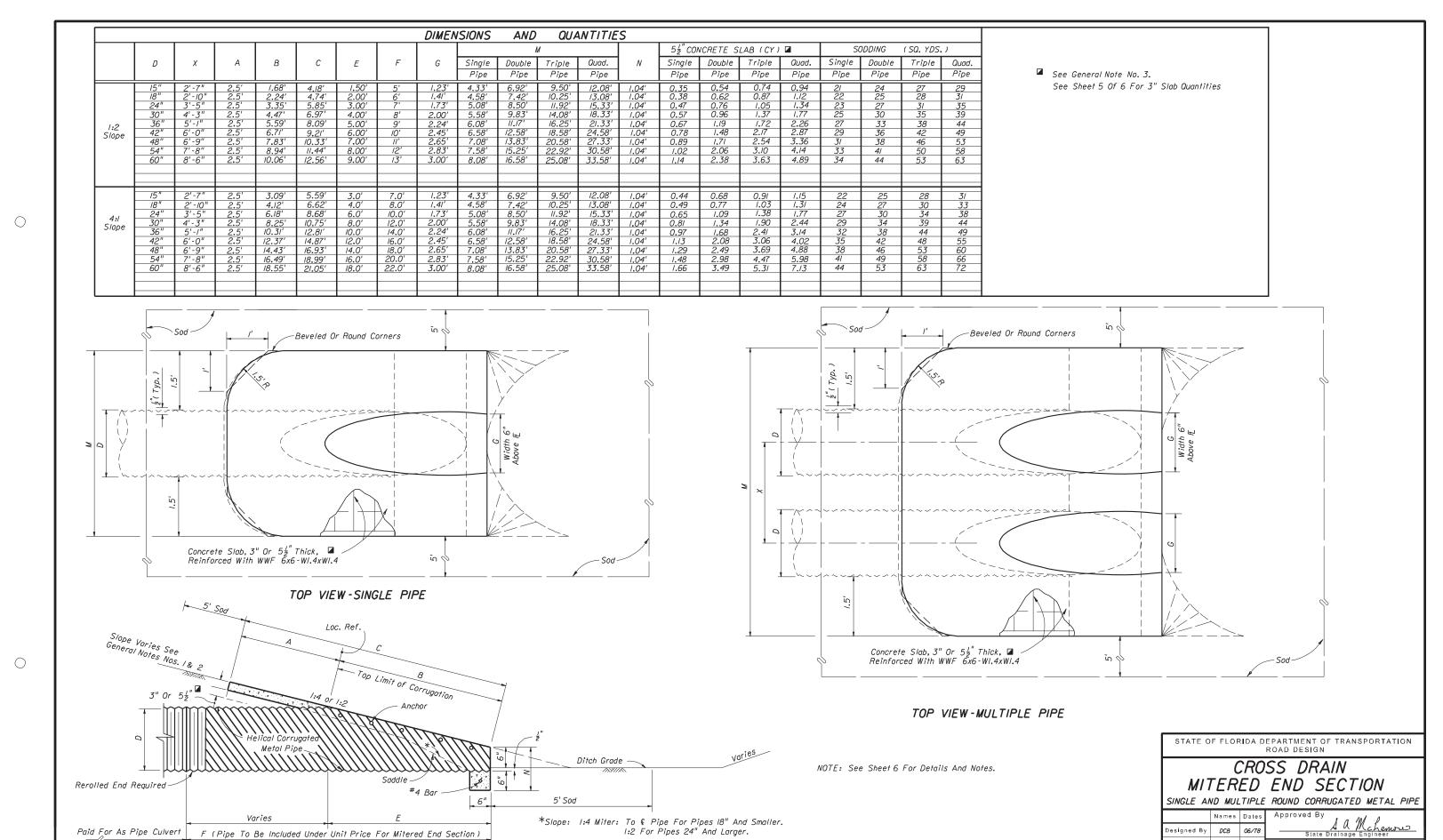
- 2. Reinforced concrete jackets shall be used at all locations where high velocities and/or highly erosive soils may cause disjointing. These locations are to be shown on the plans.
- 3. Toe walls shall be used whenever the anticipated velocity of discharge and soil type are such that erosive action would occur. Toe walls are not required where ditch pavement is provided, except when disjointing would occur if the ditch pavement should fail.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

FLARED END SECTION

	Names	Dates	Approve	d By	10
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Checked By	JVG	09/77	00	l of l	270





l:2 Miter: To € Pipe For Pipes 18" And Smaller. I:1 For Pipes 24" And Larger.

SECTION

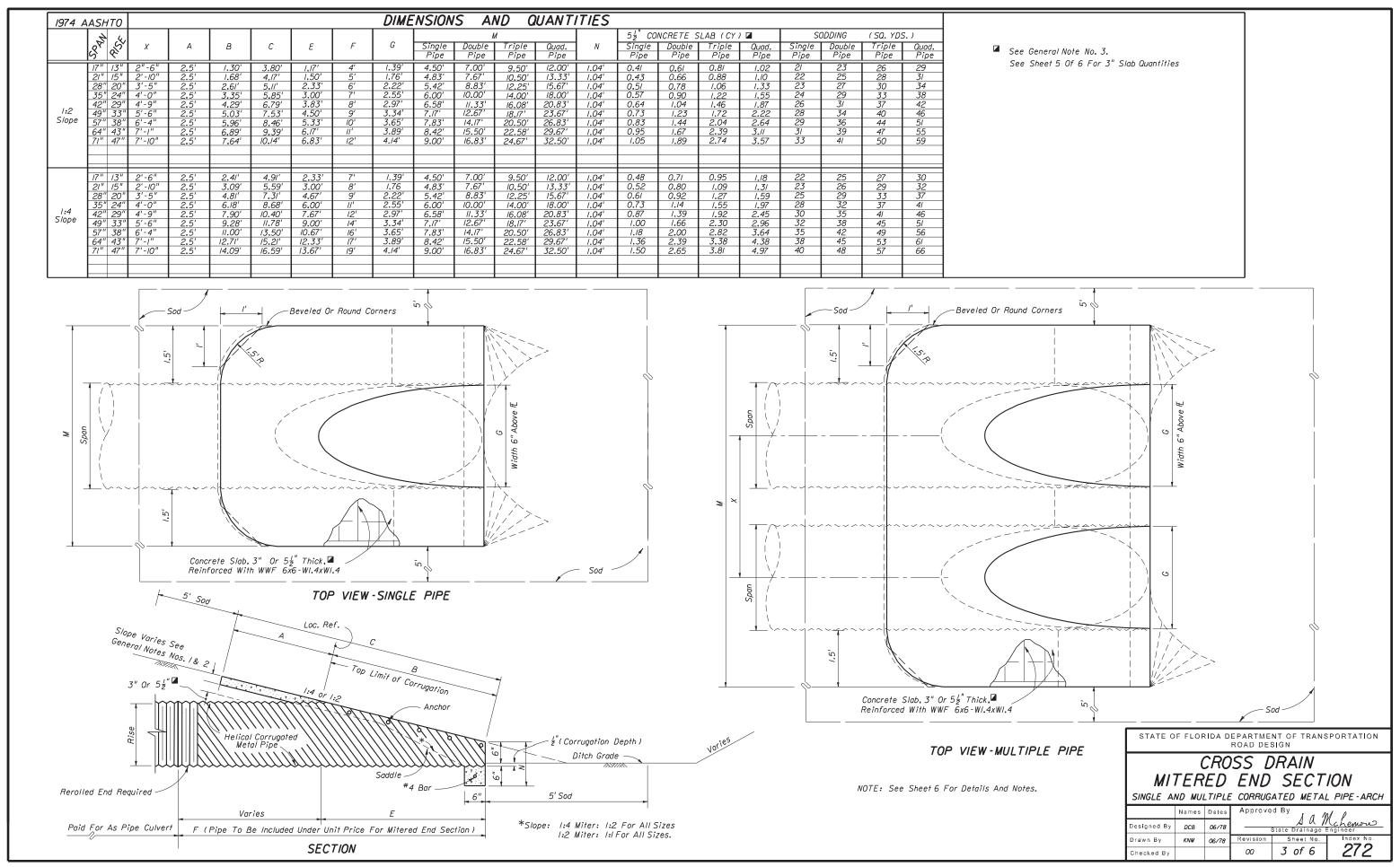
2 of 6

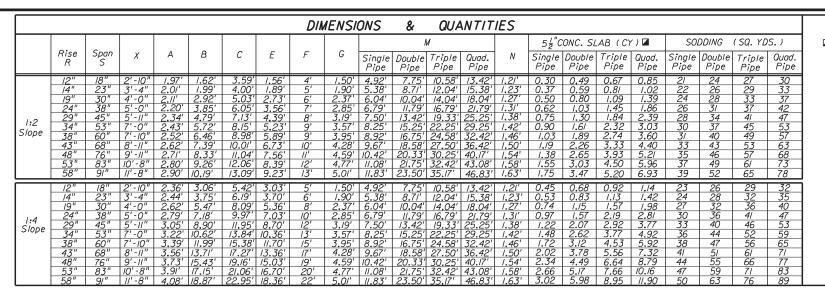
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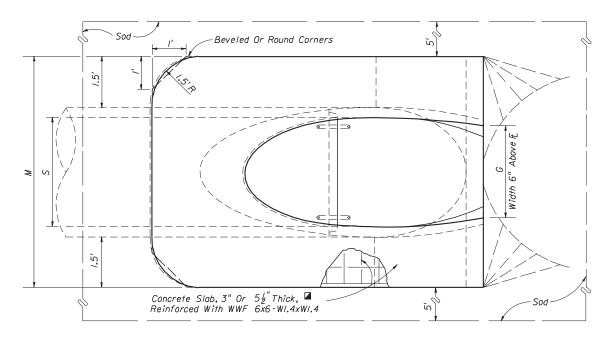
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☑ See General Note No. 3. See Sheet 5 Of 6 For 3" Slab Quantities

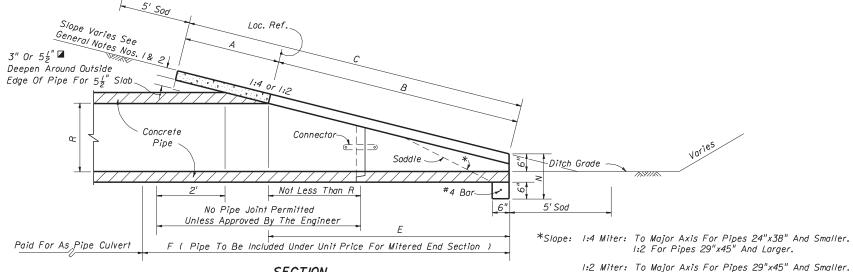


-Beveled Or Round Corners Concrete Slab, 3" Or $5\frac{1}{2}$ " Thick, Reinforced With WWF 6x6-W1.4xW1.4

TOP VIEW - MULTIPLE PIPE

NOTE: See Sheet 6 For Details And Notes.

TOP VIEW-SINGLE PIPE



I:I For Pipes 34"x53" And Larger.

SECTION

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CROSS DRAIN MITERED END SECTION

SINGLE AND MULTIPLE ELLIPTICAL CONCRETE PIPE

	Names	Dates	Approve	d By	1/ 0
Designed By	EGR	06/81		ا/ لما لم State Drainage I	Chemoro Engineer
Drawn By	HSD	06/81	Revision	Sheet No.	Index No.
Checked By	JVG/JBW	06/81	00	4 of 6	272

QUANTITIES FOR 3" THICK CONCRETE SLABS (CY)

		R	OUND - C	CONCRE	TE
	D	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	15"	0.27	0.41	0.54	0.67
	18"	0.31	0.45	0.60	0.75
	24"	0.39	0.59	0.79	1.00
	30"	0.46	0.76	1.04	1.32
, 0	36"	0.55	0.94	1.33	1.71
1:2	42"	0.66	1.15	1.66	2.15
Slope	48"	0.76	1.37	1.96	2.57
	54"	0.87	1.62	2.38	3.14
	60"	0.99	1.90	2.81	3.73
	66"	/.//	2.15	3.21	4.27
	72"	1.24	2.46	3.68	4.90
	15"	0.40	0.6/	0.80	1.00
	18"	0.47	0.69	0.9/	1.14
	24"	0.60	0.90	1.21	1.52
	30"	0.76	1.19	1.63	2.07
1.1	36"	0.89	1.48	2.05	2.63
1:4	42"	1.05	1.82	2.57	3.34
Slope	48"	1,21	2.15	3.07	4.00
	54"	1.39	2.55	3.72	4.88
	60"	1.59	3.02	4.44	5.86
	66"	1.91	3.66	5.40	7.15
	72"	2.12	4.18	6.24	8.30

			ROUN	D-CMP	
	D	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	/5"	0.24	0.37	0.5/	0.64
	18"	0.26	0.43	0.61	0.78
	24"	0.32	0.52	0.72	0.91
	30"	0.38	0.64	0.91	1.18
	36"	0.44	0.78	1.13	1,48
1:2	42"	0.51	0.96	1.41	1.87
Slope	48"	0.57	1.09	1.63	2.15
	54"	0.65	1.32	1.99	2.66
	60"	0.71	1.49	2.28	3.07
	/5"	0.31	0.47	0.63	0.79
	18"	0.34	0.53	0.71	0.90
	24"	0.44	0.69	0.92	1.18
	30"	0.53	0.88	1.25	1.60
1:4	<i>3</i> 6"	0.62	1.07	1.53	2.00
	42"	0.71	1.30	1.92	2.52
Slope	48"	0.80	1.54	2.29	3.02
	54"	0.91	1.83	2.74	3.67
	60"	1.02	2.15	<i>3,2</i> 7	4. 39

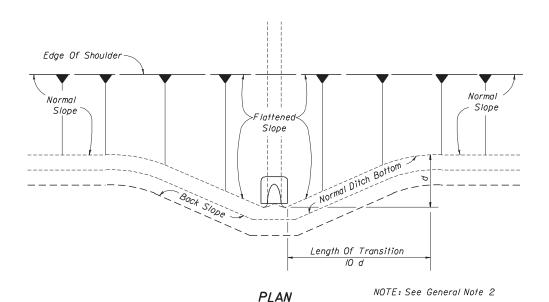
	2	e		CMP	-ARCH	
	Span	Rise	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	17"	/3"	0.33	0.49	0.65	0.81
	21"	15"	0.33	0.50	0.67	0.83
	28"	20"	0.37	0.56	0.76	0.95
	35"	24"	0.40	0.62	0.84	1.07
١,,	42"	29"	0.43	0.70	0.98	1.25
1:2	49"	33"	0.49	0.82	1.15	1.48
Slope	57"	38"	0.55	0.95	/ . 35	1.75
	64"	43"	0.62	1.10	1.57	2.05
	7/"	47"	0.69	1.24	1.80	2.35
	1711	1711	0.70	0.56	0.74	0.00
	17"	/3"	0.38	0.56	0.74	0.92
	21"	15"	0.39	0.59	0.80	0.95
	28"	20"	0.43	0.64	0.88	1.10
	35"	24"	0.49	0.77	1.05	1.33
1:4	42"	29"	0.57	0.92	1.27	1.62
Slope	49"	33"	0.65	1.08	1.50	1.93
Jope	57"	38"	0.76	1.30	1.83	2.37
	64"	43"	0.87	1.55	2.18	2.83
	71"	47"	0.95	1.68	2.43	3.17
				l		

	o	C.	ELL	IPTICAL	L - CONC	RETE
	Rise	Span	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
	12"	18"	0.19	0.33	0.45	0.57
	14"	23"	0.25	0.40	0.55	0.69
	19"	30"	0.34	0.55	0.75	0.95
	24"	38"	0.43	0.71	1.00	1,28
, ,	29"	45"	0.52	0.90	1.27	1.65
1:2	34"	53"	0.62	1.11	1.60	2.09
Slope	38"	60"	0.70	1.29	1.87	2.46
	43"	68"	0.81	1.54	2.26	2.99
	48"	76"	0.93	1.79	2.66	3.53
	53"		1.04	2.04	3.03	4.02
	58"	9/"	1.17	2.33	3.49	4.66
	12"	18"	0.30	0.45	0.6/	0.76
	14"	23"	0.36	0.56	0.76	0.95
	19"	30"	0.51	0.79	1.08	1.36
	24"	38"	0.68	1,10	1.53	1.96
1.1	29"	45"	0.86	1.45	2.04	2.63
1:4	34"	53"	1.02	1.81	2.60	3.39
Slope	38"	60"	1.18	2.14	3.10	4.05
	43"	68"	1.38	2.58	3.79	4.99
	48"	76"	1.59	3.05	4.51	5.97
	53"	83"	1.80	3.50	5.19	6.88
	58"	9/"	2.04	4.04	6.05	8.05

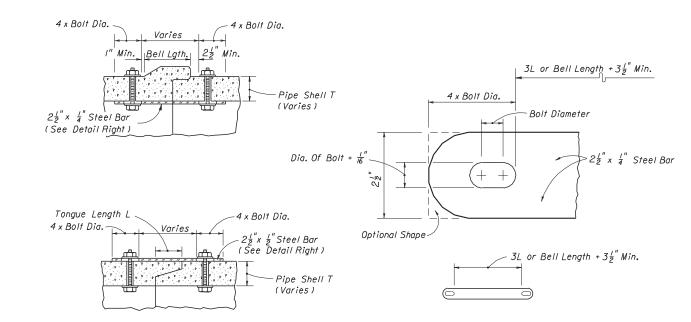
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

CROSS DRAIN MITERED END SECTION

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SLOPE AND DITCH TRANSITIONS

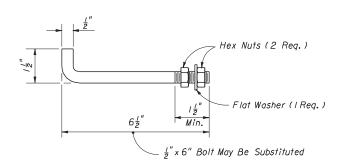


All bars, bolts, nuts and washers are to be galvanized steel. Bolts diameters shall be $\frac{3}{8}$ " for 15" to 36" pipe and $\frac{5}{8}$ " for 42" to 72" pipe. Two connectors required per joint, located 60° right and left of bottom center of pipe. Bolt holes in pipe shell are to be drilled.

CONCRETE PIPE CONNECTOR

GENERAL NOTES

- I. Mitered end sections for pipe sizes 15", 18" and 24" round or equivalent pipe arch or elliptical pipe are permitted within the clear zone. When the slope intersection permits, the mitered end section may be located with the culvert opening as close as 8' beyond the outside edge of the shoulder.
- 2. Slope and ditch transitions shall be used when the normal roadway slope must be flattened to place end section outside clear zone. See detail left.
- 3. The reinforced concrete slab shall be constructed for all sizes of cross drain pipe and cast in place with Class I concrete. Slabs shall be $5\frac{1}{2}$ thick unless 3" thickness called for in plans.
- 4. Concrete pipe used in the assembly of mitered end sections shall be selective lengths to avoid excessive connections.
- 5. Corrugated metal pipe galvanizing that is damaged during beveling and perforating for mitered end section shall be repaired.
- 6. That portion of corrugated metal pipe in direct contact with the concrete slab shall be bituminous coated prior to placing of the concrete.
- 7. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of cross drain pipe; corrugated steel pipe mitered end sections may be used with any type of cross drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of cross drain pipe except steel pipe. When bituminous coated metal pipe is specified for cross drain pipe, mitered end sections shall be constructed with like pipe or concrete pipe.
- 8. When the mitered end section pipe is dissimilar to the cross drain pipe, a concrete jacket shall be constructed in accordance with Standard Index 280.
- 9. When existing multiple cross drain pipes are spaced other than the dimensions shown in this detail, or have non-parallel axes, or have non-uniform sections, the mitered end sections will be constructed either separately as single pipe mitered end sections or collectively as multiple pipe end sections as directed by the Engineer; however, mitered end sections will be paid for each based on each independent pipe end.
- 10. The cost of all pipe(s), fasteners, reinforcing, connectors, anchors, concrete, sealants, jackets, and coupling bands shall be included in the cost for the mitered end section. Sodding shall be paid for separately under the contract unit price of Sodding, SY.
- II. Mitered end sections shall be paid for under the contract unit price for Mitered End Section (CD), Each, based on each independent pipe end.



Anchors required for CMP only.

Anchor, washer and nuts to be galvanized steel.

Bend anchor where required to center in concrete slab. Damaged surfaces to be repaired after bending. Anchors are to be spaced a distance equal to four (4) corrugations. Place the anchors in the outside crest of corrugation.

Flat washers to be placed on inside wall of pipe.

Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

ANCHOR DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

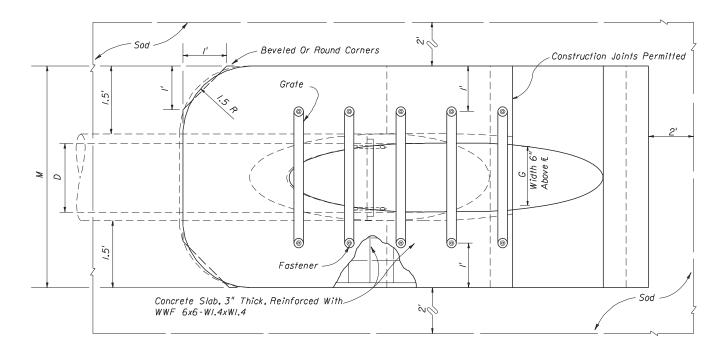
CROSS DRAIN MITERED END SECTION

SPECIAL DETAILS AND NOTES

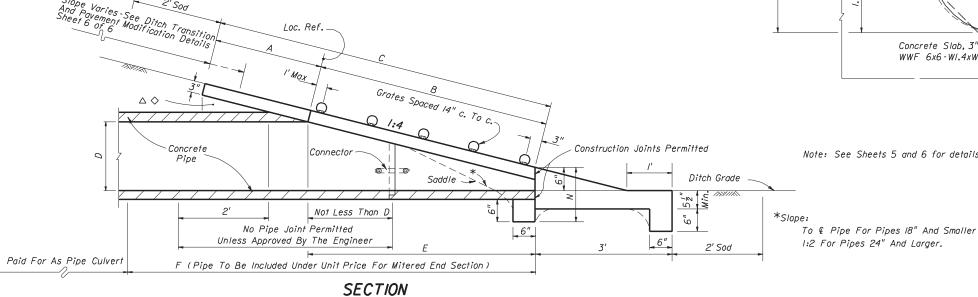
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Checked By	KNM	06/78	00	6 of 6	272

	DIMENSIONS &											QUANTIT	TIES									
									Λ	1			GRATE	SIZES	CON	CRETE	(Cu. Yo	ds.)	S	DDDING (Sq. Yds.	7
D	X	Α	В	С	E	F	G	Single	Double	Triple	Quad	Ν	Standard	Extra	Single	Double	Triple	Quad	Single	Double	Triple	Quad
								Pipe	Pipe	Pipe	Pipe		Weight Pipe	Strong Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe	Pipe
15"	2'-7"	2.27'	4.09'	6.36'	4.03'	8'	1.22'	4.63'	7.21'	9.79'	12.37'	1.19'			0.76	1.16	1.54	1.94	8	10	//	12
18"	2'-10"	2.36'	5.12'	7 .4 8′	5.03'	9'	1.41'	4.92'	7.75'	10.58'	13.42'	1.21'			0.85	1.28	1.71	2.17	9	10	12	13
24"	3'-5"	2.53'	7.18' △	9 . 71′	7.03' △	11'	1.73'	5.50'	8.92'	12.33'	15.75'	1.25'			1.02	1.58	2./5	2. 75	10	12	13	15
30"	4'-3"	2.70'	9.25'	II . 95'	9.03'	/3'	2.00'	6.08'	10.33'	14.58'	18.83'	1.29'	2 <u>!</u> "	3"	1.23	1.98	2.74	3.50	12	14	15	17
36"	5' -1"	2 . 87'	11.31'♦	14.18'	//.03'♦	<i>1</i> 5'	2.24'	6.67'	II . 75'	16.83'	21.92'	1.33'	2 <u>1</u> "	3"	1.40	2.38	3.33	4.24	/3	15	17	20
42"	6'-0"	<i>3.05</i> ′	/3.37'	16.42'	13.03'	17'	2.45'	7.25'	<i>13.25</i> ′	19.25'	25.25'	1.38'	2 <u>1</u> "	3½"	1.60	2.83	4.04	5 . 26	14	17	19	22
48"	6'-9"	<i>3,22</i> ′	15.43'	18.65'	15.03'	19'	2 . 65'	7.83'	14.58'	21.33'	28.08'	1.42'	21	31/2"	1.81	3.26	4.70	6.14	15	18	21	24
54"	7'-8"	3.39'	17.49'	20.88'	17.03'	21'	2.83'	8.42'	16.08'	23.75'	31.42'	1.46'	3"	4"	2.03	3.78	5.54	7.28	17	20	23	27
60"	8'-6"	<i>3.5</i> 6′	19.55'	23.11'	19.03'	23'	3.00'	9.00'	<i>17</i> .50′	26.00'	<i>34.</i> 50′	1.50'	3"	4"	2.28	4.36	6.43	8.50	18	22	25	29

- △ 6.42′ ♦ 10.40°
- \triangle 6.25' Dimensions permitted to allow use of 8' standard pipe lengths. \diamondsuit 10.10' Dimensions permitted to allow use of 12' standard pipe lengths.
- △ ♦ Concrete slab shall be deepened to form bridge across crown of pipe. See section below.



TOP VIEW - SINGLE PIPE



- Beveled Or Round Corners Construction Joints Permitted Fastener Concrete Slab, 3" Thick, Reinforced With -Sod WWF 6x6-W1.4xW1.4

TOP VIEW-MULTIPLE PIPE

Note: See Sheets 5 and 6 for details and general notes.

SIDE DRAIN MITERED END SECTION

SINGLE AND MULTIPLE ROUND CONCRETE PIPE

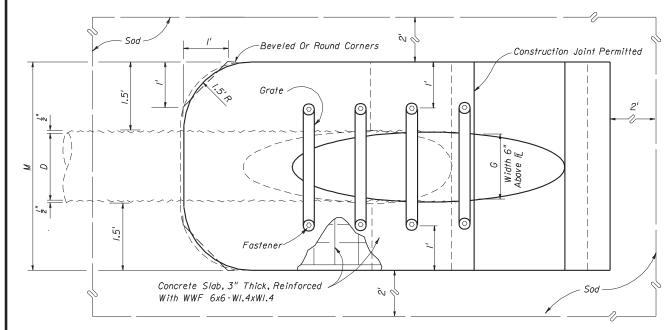
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

	Names	Dates	Approve	- 1 /1 /1//	1 p
Designed By	EGR	06/78		State Drainage E	Engineer
Drawn By	нкн	06/78	Revision	Sheet No.	Index No.
Checked By	JVG	06/78	00	I of 6	273

	DIMENSIONS & QUANTITIES																						
									I	И			GRATE	SIZES	CC	DNCRETE	(Cu.)	(ds.)		SODDING	(Sq. Yds	i.)	1
D	Х	А	В	С	Ε	F	G	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	N	Standard Weight Pipe	Extra Strong Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	
8"	2'-0"	2.5'	0.72'	3.22'	0.7'	4.0'	0.58'	3.75'	5.75'	7.75′	9.75'	1.04'			0.52	0.90	1.22	1.54	7	8	8	9	Ι,
10"	2'-2"	2.5'	1.34'	3.84'	1.3'	5.0'	0.81'	3.92'	6.08'	8.25'	10.41'	1.04'			0.64	0.99	1.34	1.70	7	8	9	10] ′,
12"	2'-4"	2 . 5'	2.06'	4.56'	2.0'	6.0'	1.00'	4.08'	6.42'	8.75'	II . 08'	1.04'			0.68	1.09	1.48	1.88	7	8	10	//	
15"	2'-7"	2.5'	3.09'	5.59'	3.0'	7.0'	1.23'	4.33'	6.92'	9.50'	12.08'	1.04'			0.64	1.00	1 .3 5	1.71	8	9	10	//	
18"	2'-10"	2.5'	4.12'	6.62'	4.0'	8.0'	1.41'	4.58'	7.42'	10.25'	13.08'	1.04'			0.69	1.09	1.49	1.89	9	10	//	12	
24"	3'-5"	2.5'	6.18'	8,68'	6.0'	10.0'	1.73'	5.08'	8.50'	11 . 92'	15.33'	1.04'			0.83	1.34	1.82	2.34	10	//	13	14	
30"	4'-3"	2.5'	8.25'	10.75′	8.0'	12.0'	2.00'	5.58'	9.83'	14.08'	<i>18.33</i> ′	1.04'	2 <u>′</u> ″	3"	0.96	1.63	2.32	2.99	//	13	15	17	1
36"	5'-1"	2.5'	10.31'	12.81'	10.0'	14.0'	2.24'	6.08'	11.17'	16.25'	21.33'	1.04'	21"	3"	1.08	1.92	2.77	3.62	12	14	17	19	
42"	6'-0"	2.5'	12.37'	14.87'	12.0'	16.0'	2.45'	6.58'	12.58'	18.58'	24.58'	1.04'	2½"	3 <u>/</u> "	1.20	2.26	3.34	4.61	13	16	18	21	1
48"	6'-9"	2.5'	14.43'	16.93'	14.0'	18.0'	2.65'	7.08'	13.83'	20.58'	27.33'	1.04'	2½"	3½"	1.60	3.11	4.62	6.12	14	17	20	23	1
54"	7'-8"	2.5'	16.49'	18.99'	16.0'	20.0'	2.83'	7.58'	<i>15.25′</i>	22.92'	30.58′	1.04'	3"	4"	1.76	3.56	5.34	7.14	15	19	22	26	1
60"	8'-6"	2.5'	<i>18.</i> 55'	21.05'	18.0'	22.0'	3.00'	8.08'	16.58'	25.08'	33.58'	1.04'	3"	4"	1.94	4.03	6.12	8.20	17	20	24	28	1

REMARKS

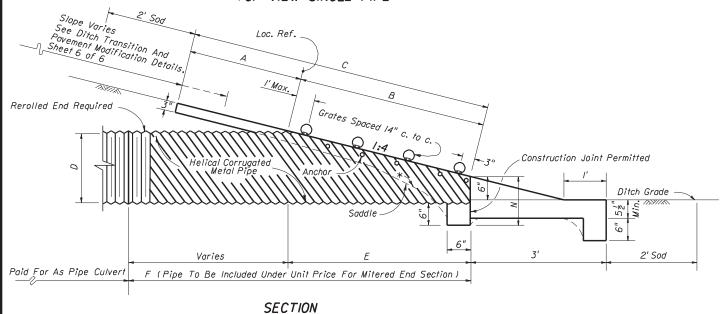
These sizes are restricted to inlet and outlet treatment for water management systems or similar applications.

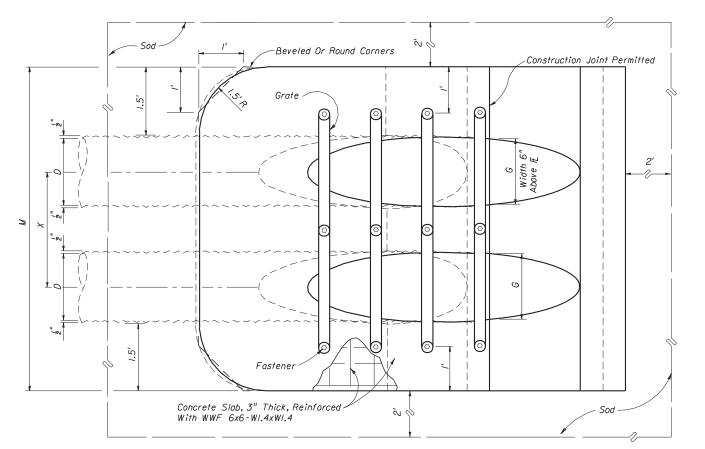


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TOP VIEW-SINGLE PIPE





TOP VIEW - MULTIPLE PIPE

NOTE: See Sheets 5 and 6 for details and general notes.

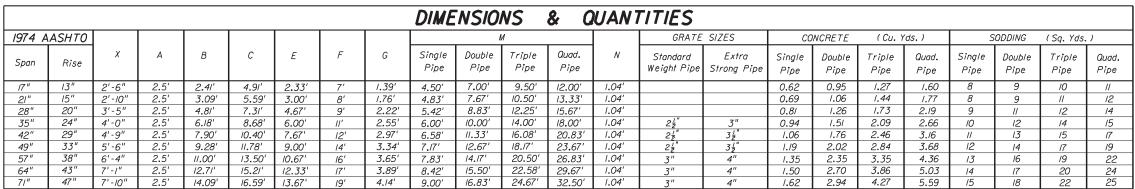
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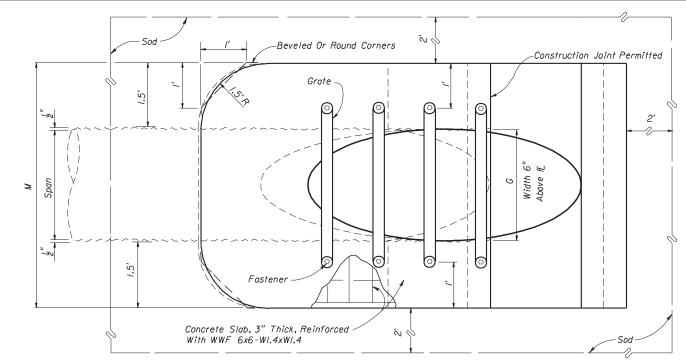
To € Pipe For Pipe 18" And Smaller I:2 For Pipe 24" And Larger STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SIDE DRAIN MITERED END SECTION

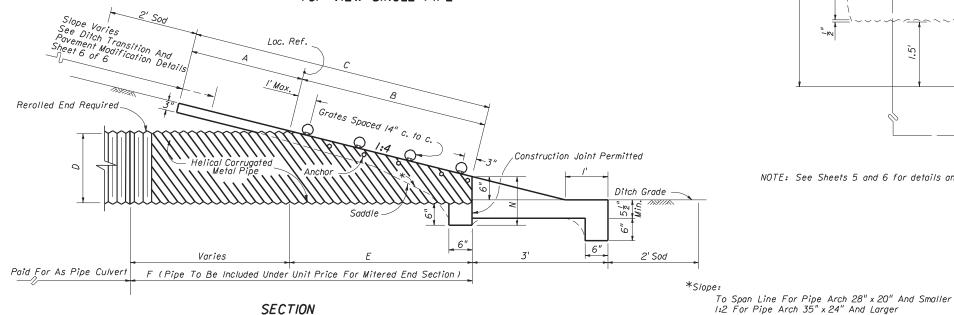
SINGLE AND MULTIPLE ROUND CORRUGATED METAL PIPE

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Checked By	JVG	08/77	00	2 of 6	l <i>273</i>

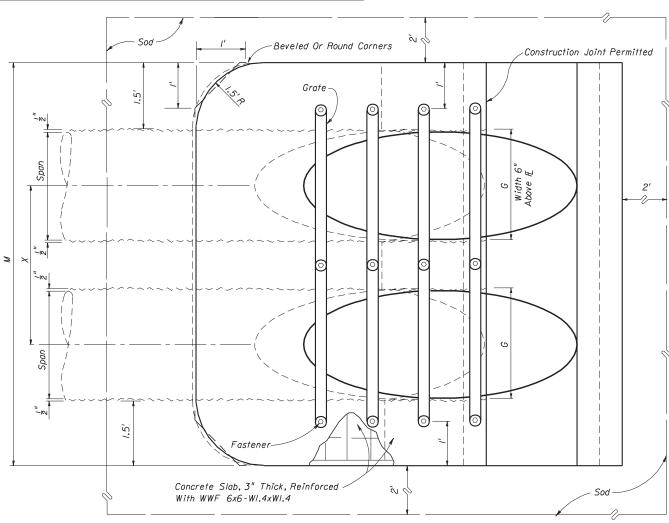




TOP VIEW-SINGLE PIPE



NOTE: See Sheets 5 and 6 for details and general notes.



TOP VIEW-MULTIPLE PIPE

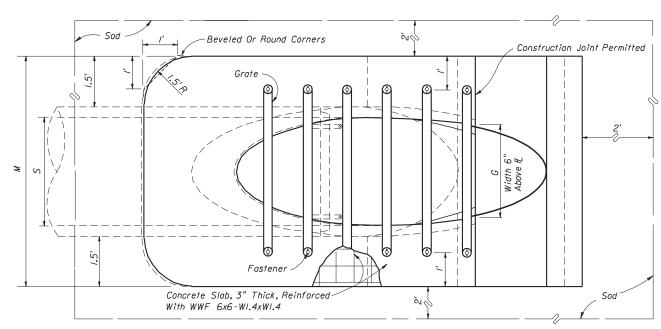
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SIDE DRAIN MITERED END SECTION

SINGLE AND MULTIPLE CORRUGATED METAL PIPE-ARCH

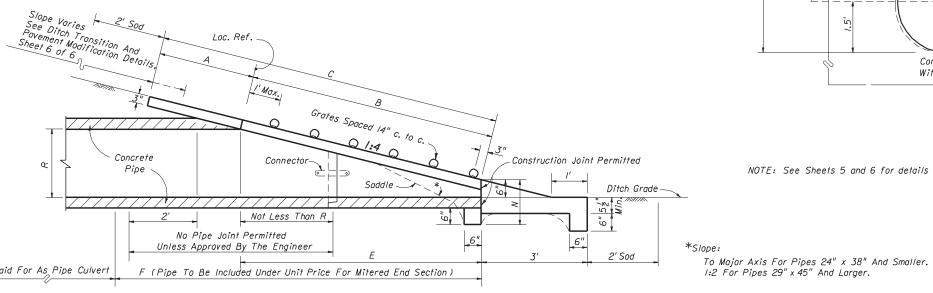
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									DI	MENS	SIONS	&	QUA	NTITIE	S								
										/	И			GRATE	SIZES	CC	NCRETE	(Cu. Y	'ds.)	S	ODDING	(Sq. Yds.	.)
Rise R	Span S	Х	Α	В	С	Ε	F	G	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	N	Standard Weight Pipe	Extra Strong Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
12"	18"	2'-10"	2.36'	3.06'	5 . 42'	3.03'	5′	/.50'	4.92'	7.75′	10.58'	13.42'	1.21'			0.68	1.04	1.41	1.77	8	9	//	12
14"	23"	3'-4"	2,44'	3.75'	6.19'	3.70'	6'	1.90'	5 . 38′	8.71'	12.04'	<i>15.38</i> ′	1.23'			0.76	1.19	1.63	2.05	9	10	12	13
19"	30"	4'-0"	2.62'	5. <i>4</i> 7'	8.09'	5.36'	8'	2.37'	6.04'	10.04'	14.04'	18.04'	1.27'	2 <u>1</u> "	3"	0.95	1.52	2.09	2.65	10	12	13	15
24"	38"	5'-0"	2,79'	7.18'	9.97'	7.03'	10'	2.85'	6.79'	11.79'	16.79'	21,79'	1.31'	2 <u>!</u> "	3"	1.18	1.95	2.74	3.53	//	13	15	18
29"	45"	5'-11"	3.05'	8.90'	// . 95′	8.70'	12'	3./9'	7.50'	13.42'	19.33'	25.25'	1.38'	2 <u>!</u> "	3₺"	1.41	2.42	3.44	4.45	12	15	18	20
34"	53"	7'-0"	3.22'	10.62'	13.84'	10.36'	13'	3. 57'	8.25'	15.25'	22,25'	29,25'	1.42'	3"	3 <u>₽</u> "	1.63	2.92	4.22	5.52	/3	17	20	23
38"	60"	7'-10"	3.39'	11.99'	15.38'	11.70'	15'	3. 95'	8.92'	16.75'	24.58'	32.42'	1 .4 6'	3"	4"	1.83	3.36	4.89	6.41	14	18	21	25
43"	68"	8'-//"	3.56'	13.71'	17.27'	/3.36'	17'	4.28'	9,67'	18.58'	27.50'	36.42'	1.50'	3"	4"	2.09	3.95	5.80	7.65	16	20	23	27
48"	76"	9'-11"	3.73'	15.43'	19.16'	15.03'	19'	4. 59'	10.42'	20.33'	30.25'	40.17'	1.54'	Special	Special	2.37	4.54	6.73	8.92	17	21	26	30
53"	83"	10'-8"	3 . 91'	17.15'	21.06'	16.70'	20'	4.77'	11.08'	21.75'	32.42'	43.08'	1.58'	Special	Special	2.61	5.09	7.56	10.03	18	23	27	32
58"	91"	11'-8"	4.08'	18.87'	22.95'	18.36'	22'	5.0/	II . 83'	23.50'	35.17'	46.83'	1.63'	Special	Special	2.91	5.77	8.64	11.50	19	24	29	35



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TOP VIEW-SINGLE PIPE



-Beveled Or Round Corners Construction Joint Permitted Fastener-Concrete Slab, 3" Thick, Reinforced With WWF 6x6-WI.4xWI.4

TOP VIEW - MULTIPLE PIPE

NOTE: See Sheets 5 and 6 for details and general notes.

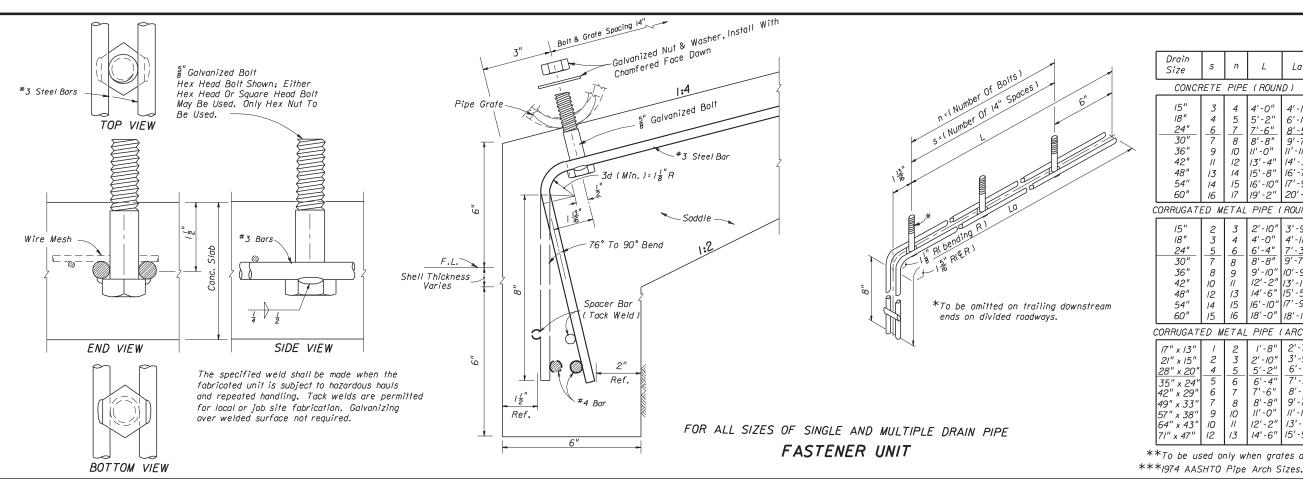
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

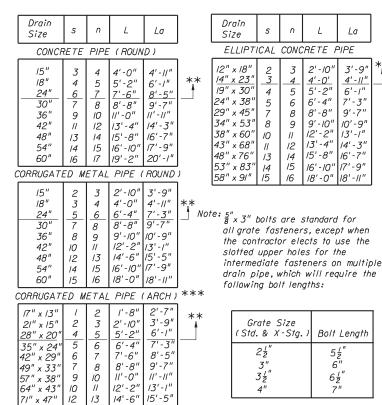
SIDE DRAIN MITERED END SECTION

SINGLE AND MULTIPLE ELLIPTICAL CONCRETE PIPE

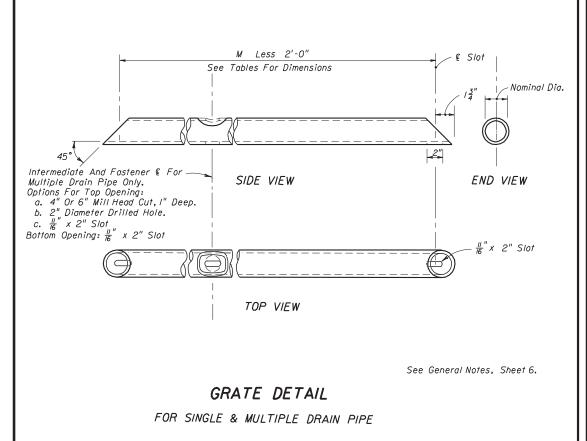
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SECTION

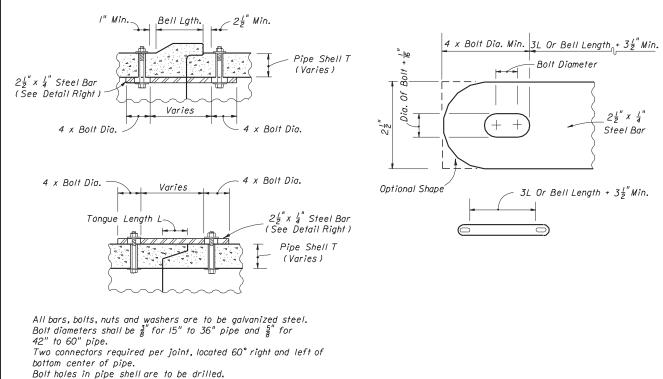




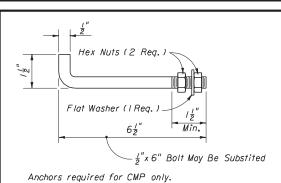
**To be used only when grates are called for in the plans.



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CONCRETE PIPE CONNECTOR DETAIL



2'-10"

4'-0'

5'-2"

6'-4"

10

/3

Grate Size

Std. & X-Stg.

3/2"

12 14 15

8'-8"

9'-10" 10'-9' 12'-2" 13'-1"

13'-4" 14'-3"

15' -8" 16' -7" 16' -10" 17' -9"

Bolt Length

6½"

3'-9"

4'-11'

6'-1'

Anchor, washer and nuts to be galvanized steel. Bend anchor where required to center in concrete slab. Damaged surfaces to be repaired after bending. Anchors are to be spaced a distance equal to four (4) corrugations. Place the anchors in the outside crest of corrugation. Flat washer to be placed on inside wall of pipe.

Holes in the mitered end pipe are to be drilled or punched; burning not permitted.

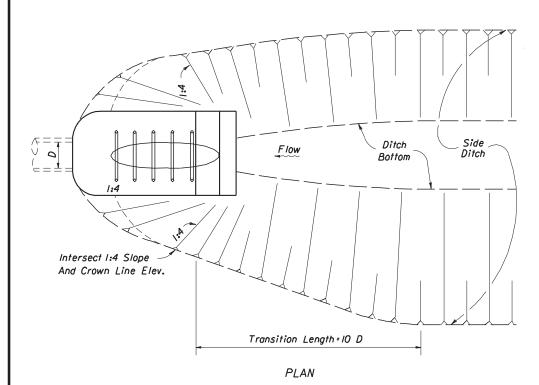
ANCHOR DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

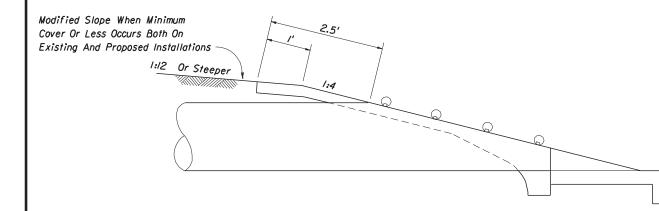
SIDE DRAIN MITERED END SECTION

DETAILS FOR CONCRETE & CORRUGATED METAL PIPE

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DITCH TRANSITION



PERMISSIBLE PAVEMENT MODIFICATION

GENERAL NOTES

- I. Unless otherwise designated in the plans, concrete pipe mitered end sections may be used with any type of side drain pipe; corrugated steel pipe mitered end sections may be used with any type of side drain pipe except aluminum pipe; and, corrugated aluminum mitered end sections may be used with any type of side drain pipe except steel pipe. When bituminous coated metal pipe is specified for side drain pipe, mitered end sections shall be constructed with like pipe or concrete pipe. When the mitered end section pipe is dissimilar to the side drain pipe, a concrete jacket shall be constructed in accordance with Index No. 280.
- 2. Concrete pipe used in the assembly of mitered end sections shall be of selective lengths to avoid excessive connections.
- 3. Corrugated metal pipe galvanizing that is damaged during beveling and perforating for mitered end section shall be repaired.
- 4. That portion of corrugated metal pipe in direct contact with the concrete slab shall be bituminous coated prior to placing of the concrete.
- 5. Corrugated polyethylene pipe (CPE) for side drain application of 15", 18" or 24" diameter shall utilize either corrugated metal or concrete mitered end sections. When used in conjunction with corrugated metal mitered end sections, connection shall be by either a formed metal band specifically designated to join CPE pipe and metal pipe or other coupler approved by the State Drainage Engineer. When used in conjunction with a concrete mitered end section, connection shall be by concrete jacket constructed in accordance with Index No. 280.
- 6. When existing multiple side drain pipes are spaced other than the dimensions shown in this detail, or have non-parallel axes, or have non-uniform sections, the mitered end sections will be constructed either separately as single pipe mitered end sections or collectively as multiple pipe end sections as directed by the Engineer; however, mitered end sections will be paid for each, based on each independent pipe end.
- 7. In addition to the requirements of Section 430-4, side drain culverts shall comply with the cover requirements shown on Index No. 205.
- 8. The reinforced concrete slab shall be constructed for all sizes of side drain pipe and cast in place with Class I concrete.
- 9. Round pipe size 30" or greater, pipe-arch size 35" x 24" or greater and elliptical pipe 19" x 30" or greater shall be grated unless excepted in the plans. Smaller sizes of pipe shall be grated only when called for in plans. The lower grate on trailing downstream ends on divided highways shall be omitted.
- IO. Grates are to be fabricated from steel ASTM A53, Grade B, pipe. The lower grate on all traffic approach ends shall be Schedule 80 and all remaining grates shall be Schedule 40.
 Grates subject to salt free and corrosive free environment may be fabricated from galvanized pipe, with base metal exposed during fabrication repaired as specified in Section 562, Standard Specifications; or, fabricated from black pipe and hot dipped galvanized after fabrication in accordance with ASTM Al23.
 Grates subject to salt water or highly corrosive environment shall be hot dipped galvanized after fabrication in accordance with ASTM Al23.
- II. Ditch transitions shall be used on all grades in excess of 3% as directed by the Engineer.
- 12. The project engineer shall contact the District Drainage Engineer for possible alternate treatment prior to constructing side drain mitered end sections where a minimum spacing of 30' will not result between the toe points of the mitered end sections.
- 13. The cost of all pipe (s), grates, fasteners, reinforcing, connectors, anchors, concrete, sealants, jackets and coupling bands shall be included in the cost for the mitered end section. Sodding shall be paid for separately under the contract unit price for Sodding, SY.
- 14. Mitered end sections shall be paid for under the contract unit price for Mitered End Section (SD), Ea., based on each independent pipe end.

DESIGN NOTES

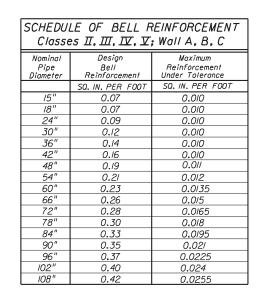
- I. In critical hydraulic locations, grates shall not be used until potential debris transport has been evaluated by the drainage engineer and appropriate adjustments made. Ditch grades in excess of 3% or pipe with less than 1.5' of cover and grades in excess of 1% will require such an evaluation (General Note 9).
- 2. The design engineer shall determine highly corrosive locations and specify in the plans when the grates shall be hot-dipped galvanized after fabrication (General Note 10).
- 3. The design engineer shall determine and designate in the plans which alternate types of mitered end section will not be permitted. The restriction shall be based on corrosive or structural requirements.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

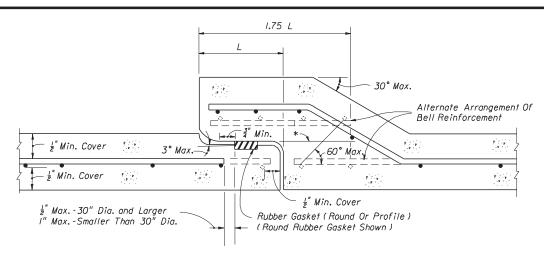
SIDE DRAIN MITERED END SECTION

NOTES & INFORMATION

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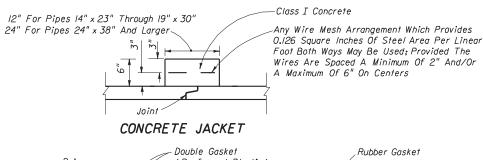
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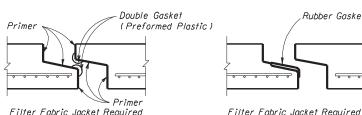


*All circumferential steel located above this line within 1.75 L is defined as bell reinforcement.

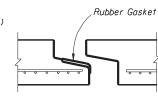
ROUND RUBBER GASKET SHOWN

DETAIL OF BELL & SPIGOT CONCRETE PIPE JOINT USING ROUND OR PROFILE RUBBER GASKET





Filter Fabric Jacket Required PREFORMED PLASTIC JOINT (BEFORE PULL-UP)

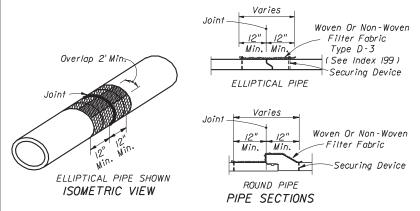


Filter Fabric Jacket Required

PROFILE RUBBER GASKET (BEFORE PULL-UP)

Cost of concrete jacket or filter fabric jacket to be included in cost of elliptical concrete pipe culverts.

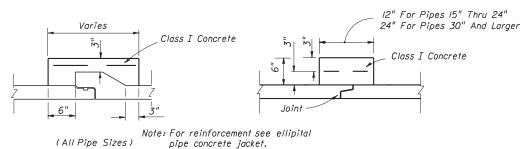
ELLIPTICAL CONCRETE PIPE JOINTS



Cost of filter fabric jacket to be included in cost of pipe culverts.

FOR ALL PIPE TYPES - CONCRETE PIPE SHOWN

FILTER FABRIC JACKET

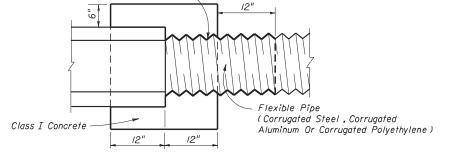


TONGUE & GROOVE

DISSIMILAR JOINTS

Bituminous Coating Required For CMP (Any Suitable Bituminous Material May Be Field Applied

BELL AND SPIGOT

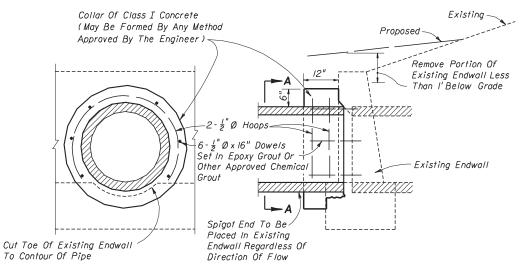


Note: Cost of concrete and bituminous coating to be included in contract unit price for either new pipe or Mitered End Section. A concrete jacket shall not be used to join: (a) metal pipe of dissimilar materials

(b) flexible pipe when the minimum cover required in accordance with Index No. 205 cannot be obtained.

DISSIMILAR TYPES

CONCRETE JACKET FOR CONNECTING DISSIMILAR TYPES OF PIPE AND CONCRETE PIPES WITH DISSIMILAR JOINTS

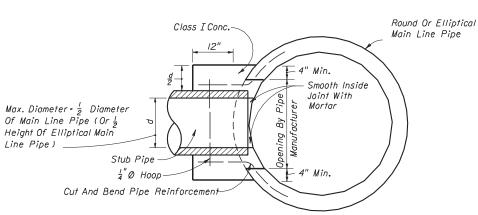


SECTION AA

LONGITUDINAL SECTION

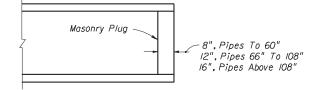
Note: Cost for removal and disposal of portions of top and toe of existing endwall and cost of concrete, reinforcing steel and construction of collar to be included in the contract unit price for pipe culvert.

CONCRETE COLLAR FOR EXTENSION OF EXISTING PIPE CULVERTS



Cost of concrete and steel to be included in contract unit price for pipe culvert.

CONCRETE COLLAR FOR JOINING MAINLINE PIPE AND STUB PIPE



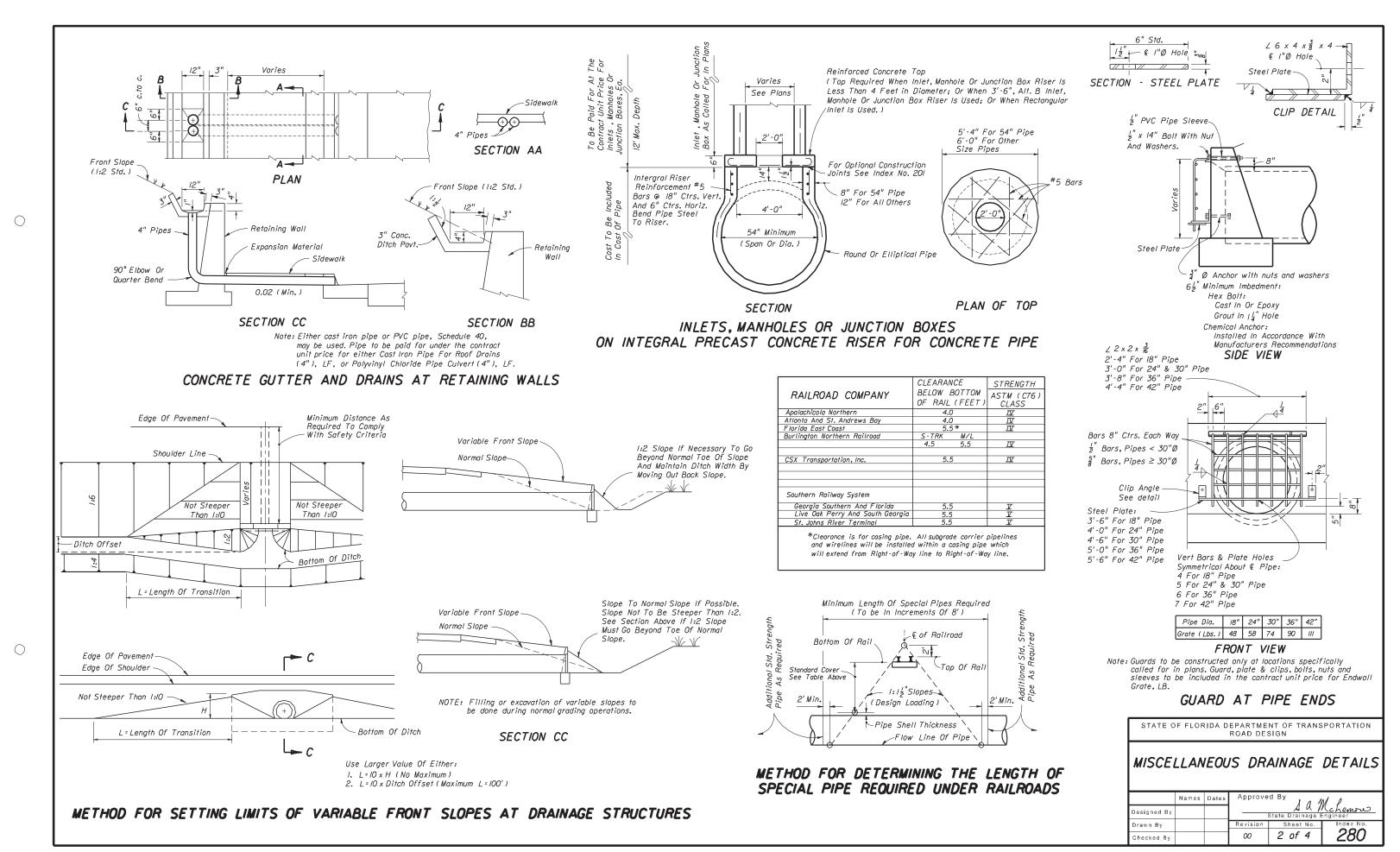
Note: Unless otherwise called for in the plans, the cost of plugging pipes to be included in contract unit price for new pipe.

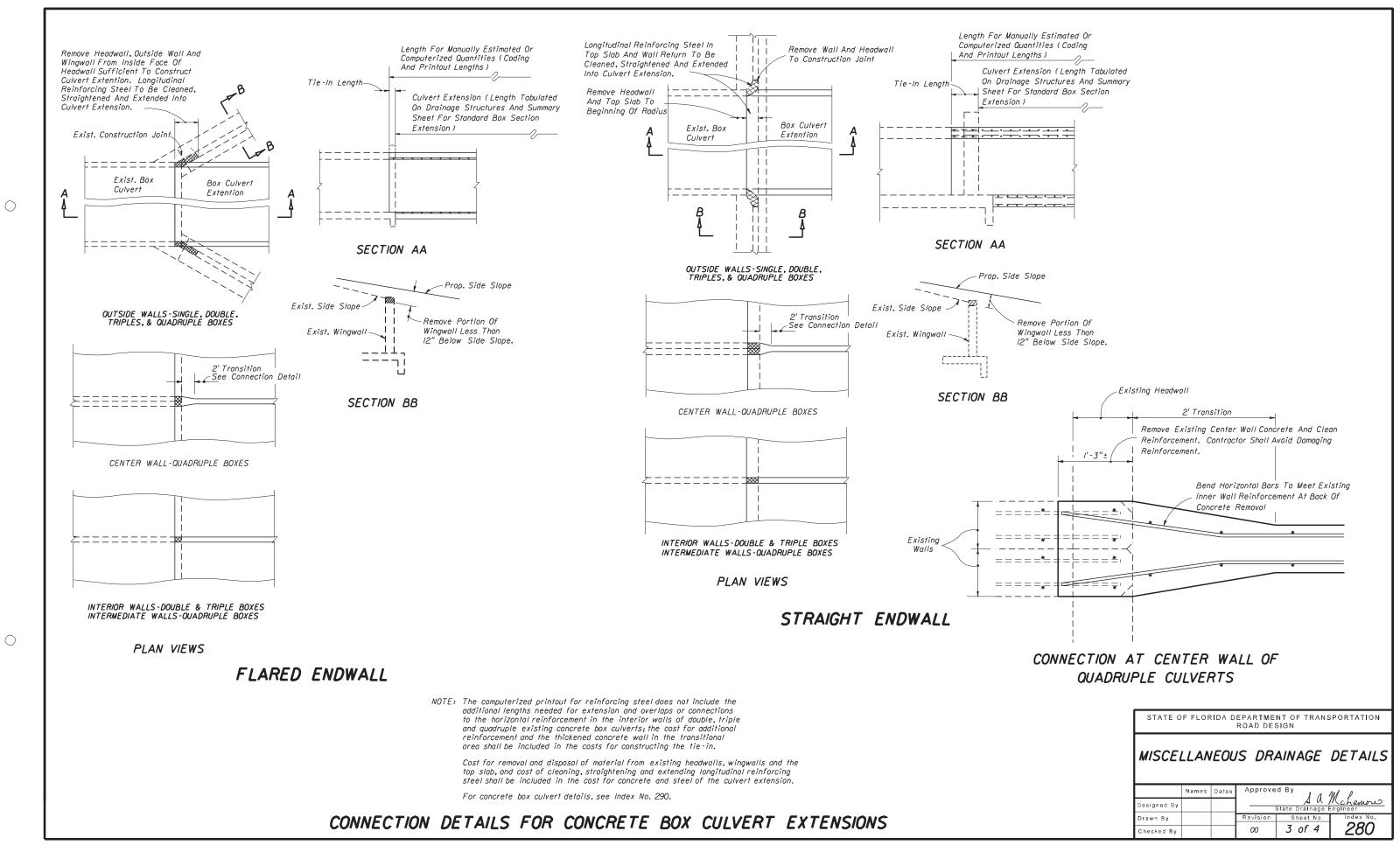
PIPE PLUG

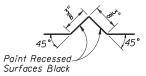
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

MISCELLANEOUS DRAINAGE DETAILS

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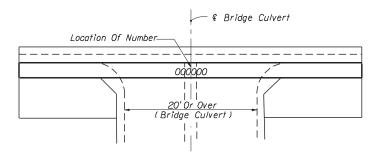






Black Plastic Figures 3" in height as approved by the Engineer may be used in liew of numbers formed by $\frac{3}{8}$ " "V" Grooves. "V" Grooves shall be formed by preformed figures.

SECTION THRU RECESSED "V" GROOVE TO FORM INSCRIBED FIGURES

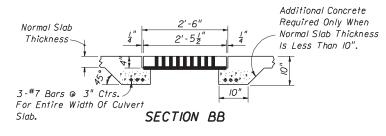


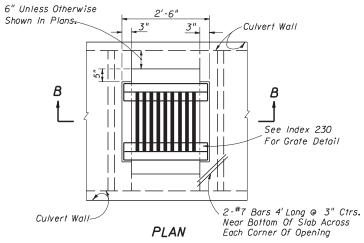
The number is to be placed in the center of the top surface of all bridge culvert headwalls.

For Bridge Number See Plan-Profile Sheet(s).

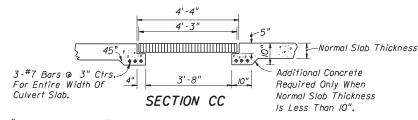
TOP VIEW OF HEADWALL

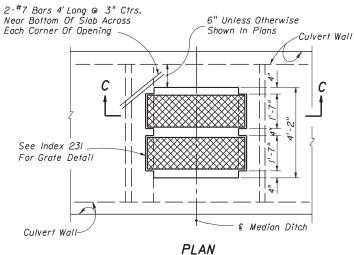
BRIDGE CULVERT NUMBER LOCATION





INLET TYPE A GRATE



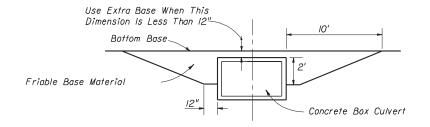


INLET TYPE B GRATE

NOTE: I. Cost of Steel Grating to be included in cost of Box Culvert.

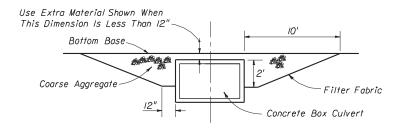
2. All steel shall be l_4^{II} clear.

INLET IN TOP OF BOX CULVERT



The cost of furnishing and installing extra friable base material shall be included in the cost of the Box Culvert.

FRIABLE BASE



The coarse aggregate shall be placed in 6 inch lifts and compacted sufficiently as to be firm and unyielding. The coarse aggregate shall be gravel or stone meeting the requirements of Section 901-2 or 901-3 respectively. The gradation shall meet Section 901-6, Grades 4, 467, 5, 56, or 57 unless restricted in the plans, the filter fabric shall be Type D-3 (See Index 199). The cost of furnishing and installing the course aggregate and filter fabric shall be included in the cost of the Box Culvert.

ASPHALTIC CONCRETE BASE

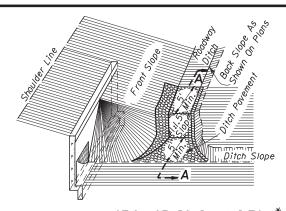
NOTE: Extra base is required when cross box culverts are located on facilities subject to high speed traffic ▷ 45 mph) or high traffic volumes (>1600 ADT) and the cover is within the range specified in the notation above.

EXTRA BASE FOR CROSS BOX CULVERTS UNDER FLEXIBLE PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

MISCELLANEOUS DRAINAGE DETAILS

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1:1.5 Slope

Lateral Ditch Grade

Do Not Construct Weep Holes In This Area Or 5' Upstream

Ditch Grade -

Varies

SECTION AA

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JUNCTION OF ROADWAY DITCH * AND LATERAL DITCH

Ditch Grade

PROFILE OF DITCH PAV'T AT LOCATIONS

Soil cement or misc. asphalt will not be permitted for this type of construction

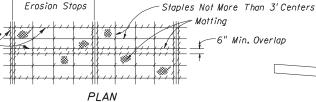
OTHER THAN JUNCTION WITH LATERAL DITCH

Shoulder Point

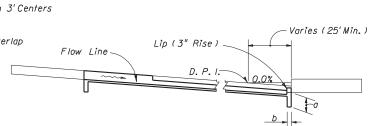
Roadway Side Slope-

One Row Of Staples And On Outer Edges At Not More Than

Each Edge Of Overlaps, Each Side Of Stops 18" Centers (Typical)



Matting-



SECTION EE

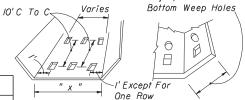
6" Typical LONGITUDINAL SECTION Mattina Side Slope ∠6" Overlap SECTION

50' Max.

MATTING FOR DITCH

Standard Paved Ditch Sodded Ditch 1.5' Deep Paved Ditch Sodded Ditch I.O' Deep I.O' Deep Sod. When Width Is Greater Than PLAN 4', Const. Weep Holes Half-

Way Up The Side In Line With PAVED DITCH END TREATMENT



When "X" = I' To 4' Const. I Row (Centered) "X" = 5' To 7' Const. 2 Rows "X" = 8' To 12' Const. 3 Rows

"X" = 13' To 17' Const. 4 Rows "X" = 18' To 22' Const. 5 Rows GENERAL NOTES

I. Type of ditch pavement shall be as shown on plans.

2. In concrete ditch pavement, contraction joints are to be spaced at 25' maximum intervals, or as directed by the Engineer. Contraction joints may be either formed (construction joint) or tooled. No open joints will be permitted.

Expansion joints with 2 preformed joint filler shall be constructed at all inlets, endwalls, and at intervals of not more than 200'.

- 3. Lip at end of ditch pavement shall normally be located downstream of D.P.I. or on flatter grades where there is a decrease in ditch velocity.
- 4. Toewalls are to be used with all ditch paving. A toewall is not required adjacent to drainage structures.
- 5. When directed by the Engineer, weep hole spacing may be reduced to 5'
- 6. For junction of R/W ditch spillway and lateral ditch, sides of paving to be I' high minimum.
- 7. For ditch pavements requiring filter fabric the fabric shall be placed directly beneath the pavement for the entire length and width of the pavement. When weep holes with aggregate are used the filter fabric shall be placed below the aggregate to form a mat continuous with or underlapping the pavement fabric. (See Index 199 for fabric type and application).
- 8. Ditch pavement requiring reinforcement shall be detailed in the plan.
- 9. Cost of plastic filter fabric to be included in the contract unit price for ditch pavement.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

DITCH PAVEMENT & SODDING

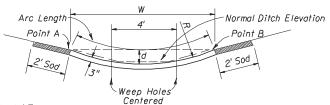
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Designed by				State Drainage E	Engineer
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DITCH PAVEMENT Filter Fabric Basis Of Dimensions Payment Velocity Pavement Type References & Remarks Range Únit Estimate Type Section 524 of the Standard Specifications. Concrete D-6 Low-High Section 339. Miscellaneous Asphalt Low - Moderate 0.2 TN/SY None SY Section 170. Soil-Cement None Low 24" | 12" | 4" Section 530. Grouting of joints required. Riprap (Sand-Cement) O.II CY/SY Low - Moderate D-4 Riprap (Ditch Lining) ΤN ΤN Moderate-High D-2

> hole. $\frac{1}{2}$ cu. ft. (12" x 12" x 6") of No. 6 aggregate to be placed under each hole. Isq. ft. of galvanized wire mesh $(\frac{1}{4}^{"})$ openings) shall be placed between the aggregate and the concrete. Cost of holes, aggregate and wire mesh to be included in the cost of ditch

WEEP HOLE ARRANGEMENT

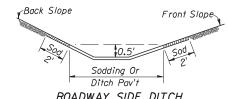
Notes: All weep holes to be 3" x 4" rectangle or 4" or 5" dia. circular



TO REPLACE: No. Of Rows Of Weep Holes Length 6' Median Swale .24' 6.0 I:6 Front Slopes; I:4 Back Slope .67 10.1 5' B.W. Ditch 4' B.W. Ditch 19' 9.1 I:4 Front Slopes & Back Slope 5' B.W. Ditch 14' 9.2 4' B.W. Ditch ((in center) 8./

ALTERNATE DITCH PAVEMENT

For use only where side slopes are 1:4 or flatter. Point "A" and "B" are to be the same elevation and should be used to locate the paved section.



Lateral Ditch 📉 🚣 A

JUNCTION OF R/W DITCH*

AND LATERAL DITCH

Front And Back Slopes Vary

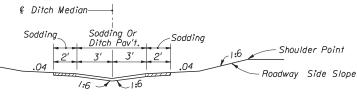
Ditch Width Varies

0.5

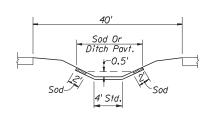
TYPICAL SECTION

Normal Ditch Elev.

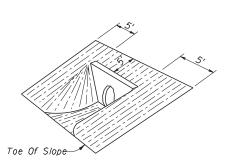
ROADWAY SIDE DITCH



SWALED MEDIAN (No Weep Holes)



40' MEDIAN

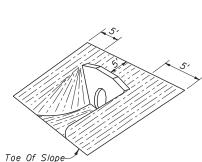


Note: Sodding quantities for each endwall to be determined by the designer from this detail.

(EXCEPT INDEX 250)

STRAIGHT ENDWALL

U - TYPE WINGS

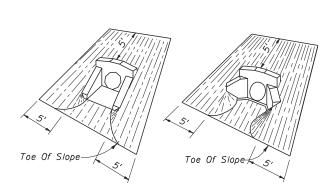


STRAIGHT ENDWALL

INDEX 250

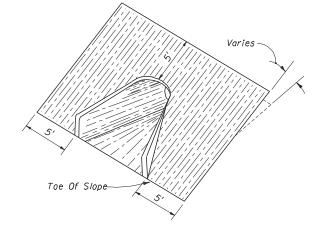
Toe Of Slope 5.

U - TYPE ENDWALL INDEX 261



WINGED ENDWALLS
INDEX 266

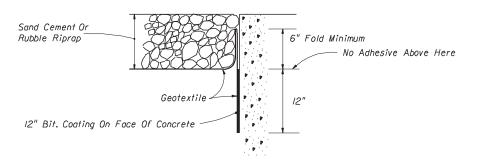
45° WINGS



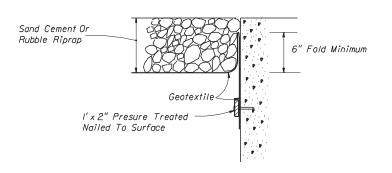
FLARED END SECTION INDEX 270

									S	ODDI	NG C	QUAN	TITIES	15	. Y. J	ı					
						INDE	X 25	0					1	NDEX	261			INDEX	(266		INDEX 270
 PIPE						SLC	PE							SL0	PE			SL	OPE		ALL SLOPES
		1: 2			1: 3			1: 4			1: 6	5	1:2	1: 3	1: 4	1: 6	1: 2	1: 3	1: 4	1: 6	020. 20
SIZE						PIP	ES							PIP	ES			PI	PES		PIPES
	1	2	3	1	2	3	1	2	3	1	2	3	1	1	1	/	1	1	1	1	1
12"																	14	15	18	22	10
<i>1</i> 5"	19	21	24	22	26	29	26	30	33	34	38	43	13 (15)	16	17	23	15	17	20	25	11
18"	21	24	27	25	29	33	30	34	38	39	44	50	14 (16)	17	19	<i>2</i> 5	16	18	22	28	11
21"																					12
24"	26	30	34	32	37	42	38	44	50	50	58	66	15 (17)	19	21	28	19	22	26	34	14
27"																					<i>1</i> 5
30"	31	37	42	39	46	53	46	55	63	62	74	85	17 (18)	21	24	32	21	25	30	40	<i>1</i> 6
36"	37	44	52	46	56	65	56	67	79	76	91	107					24	29	35	47	18
42"	43	53	62	55	67	79	67	82	96	91	111	132					27	32	39	54	19
48"	50	62	73	64	79	93	78	97	115	108	133	158					30	36	44	6/	21
54"	57	71	85	74	92	110	91	113	/36	126	157	188									21
60"																					22
66"																					25
72"																					26
	() Endwall With Baffles																				

SODDING



BONDED OPTION



NAILED OPTION

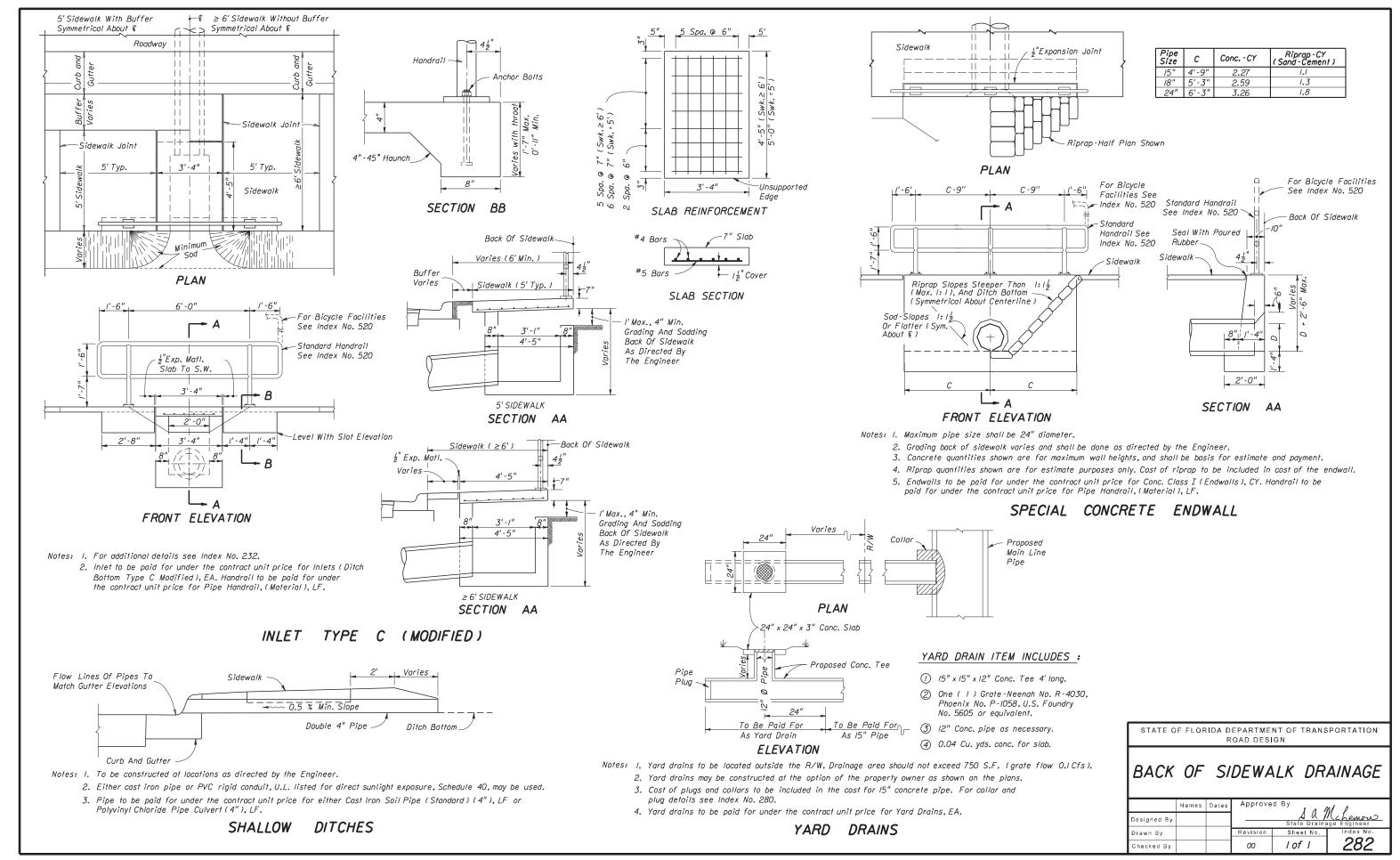
Note: Either option maybe used unless otherwise called for in the plans.

GEOTEXTILE PLACEMENT AT CONCRETE STRUCTURE

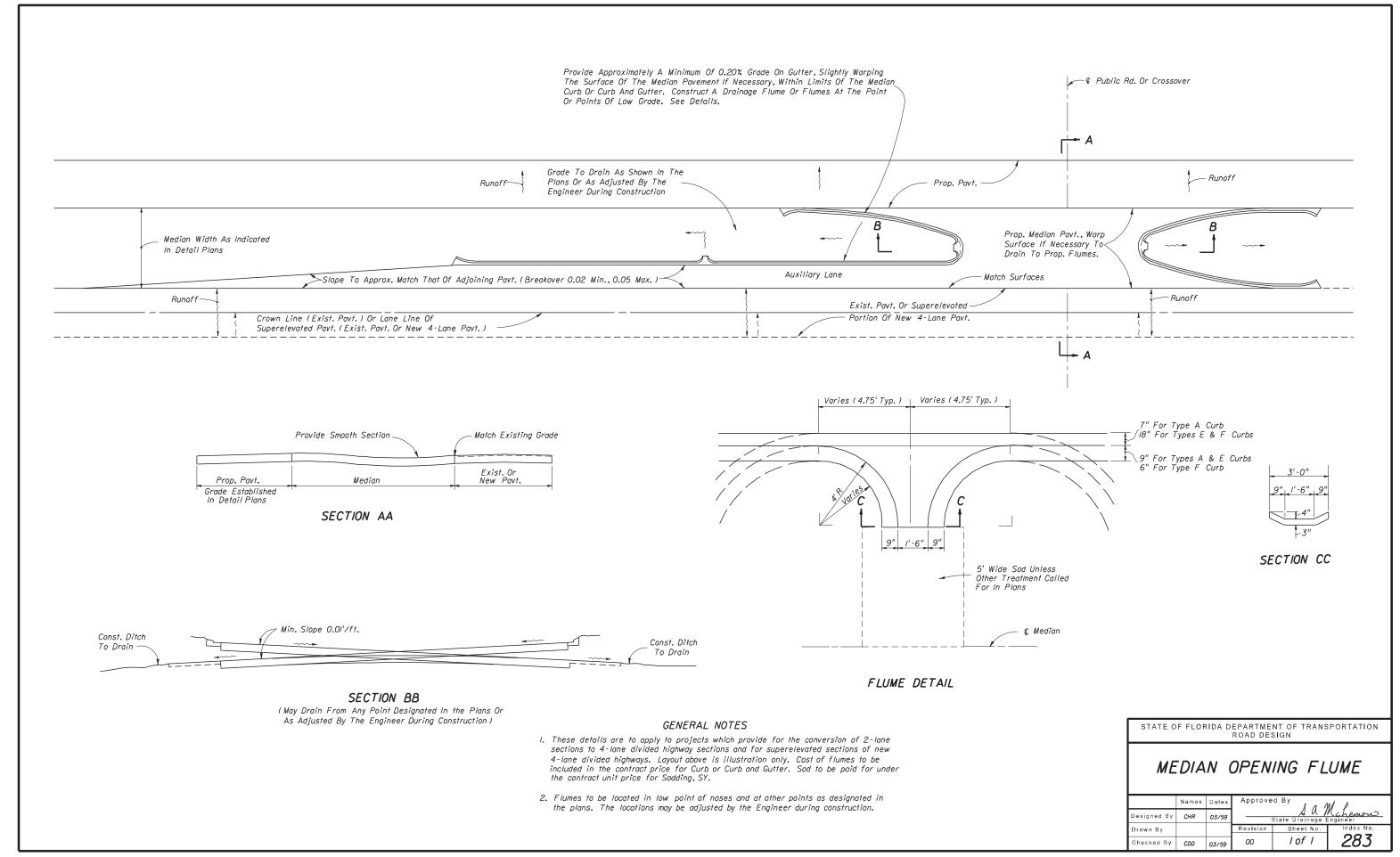
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

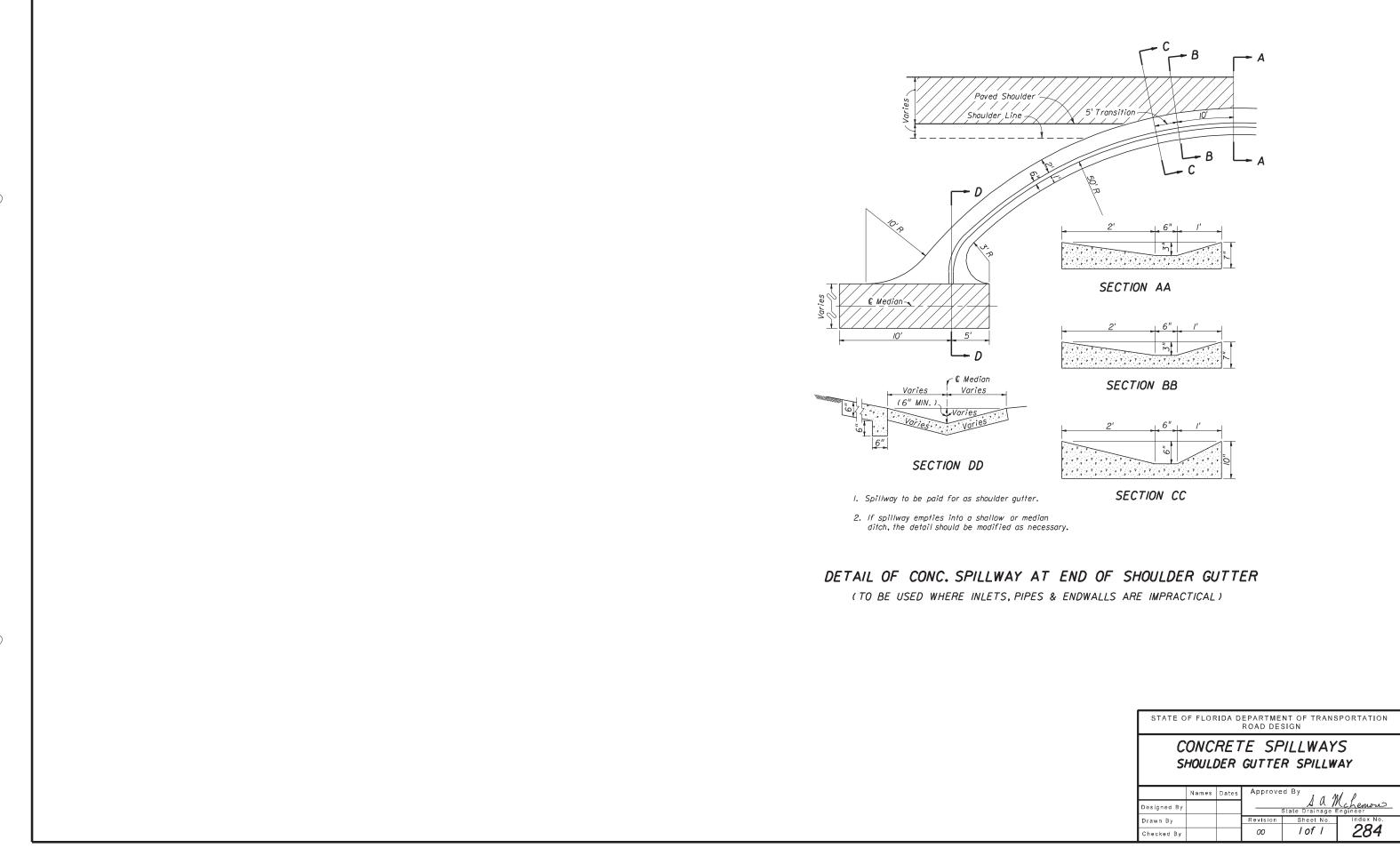
DITCH PAVEMENT & SODDING

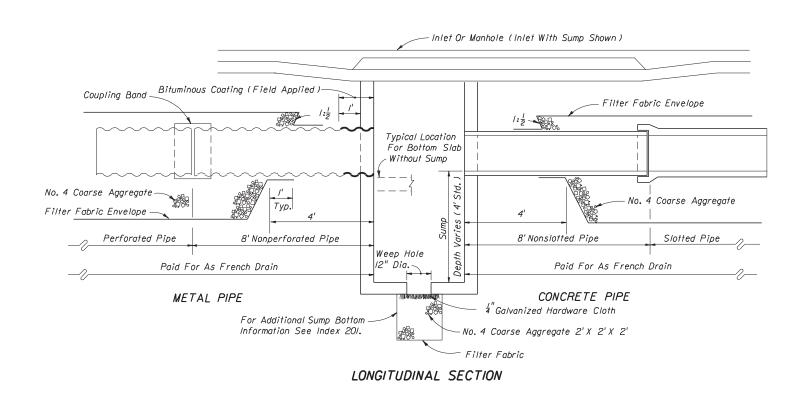
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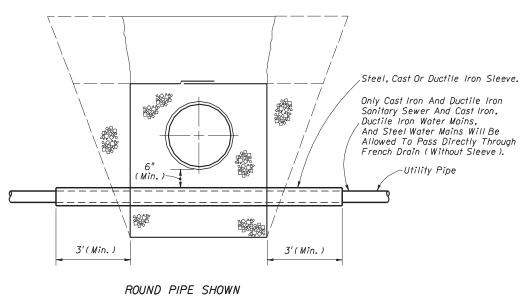


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UTILITY PIPES THRU FRENCH DRAIN

Overlap
I' Min.

Filter Fabric
Envelope

No. 4 Coarse
Aggregate

No. 4 Coarse
Aggregate

I' Std.

Pipe O. D. I' Std.

ROUND PIPE SHOWN

STANDARD CROSS SECTION (ENLARGED)

FRENCH DRAIN SYSTEM

GENERAL NOTES

- I. Pipe shall be any of the optional types permitted in Section 443 of the Specifications unless otherwise restricted in the plans. Dissimilar types of pipe will not be permitted in a continuous run of pipe.
- 2. Concrete pipe shall be placed with the slots positioned on sides.
- 3. Alignment joints are standard (gaskets not required). Recorrugation of metal pipe ends not required.
- The contractor may submit other methods of providing slots having equal or greater area of opening, for approval by the Engineer.
- 5. Filter fabric shall be Subsurface Drainage type meeting the requirements of Section 985. All filter fabric joints shall lap a minimum of one (I) foot.
- 6. The standard cross section shall be constructed unless other section(s) described or detailed in the plans.
- 7. For supplemental details see Index No. 280.
- 8. The contractor shall take the necessary precautions to prevent contamination of the trench with sand, silt and foreign materials.
- 9. The 12" diameter weep hole shall be eliminated, when the bottom of the inlet is below the normal water table, unless otherwise shown in the plans.
- 10. French drains following the typical cross section shall be paid for under the contract unit price for French Drains, LF. The unit price shall include the cost of pipe, pipe plugs, pipe fittings, coarse aggregate and filter fabric in place, and the cost for trench excavation, backfill and compaction. The unit price shall also include the cost for disposal of surplus excavated materials and cost for restoration of pavement removed or damaged by french drain construction, but shall not include payments for items paid for elsewhere.

French drains with a significantly different cross section shall be paid for under the contract unit prices for separate items as follows:

- (a) Slotted or Perforated Pipe Culvert, LF. Unit price shall include cost for pipe, pipe plugs and fittings in place.
- (b) Ballast Rock (French Drain Aggregate), CY. Unit price shall include cost for coarse aggregate in place, and cost for trench excavation, backfill and compaction. The unit price shall also include the cost for disposal of surplus excavated materials and cost for restoration of pavement removed or damaged by french drain construction, but shall not include payment for items paid for elsewhere.
- (c) Plastic Filter Fabric (Subsurface), SY. Unit price shall be for cost of fabric in place. Quantity shall be determined by plan neat dimensions of the fabric envelope.

DESIGN NOTES

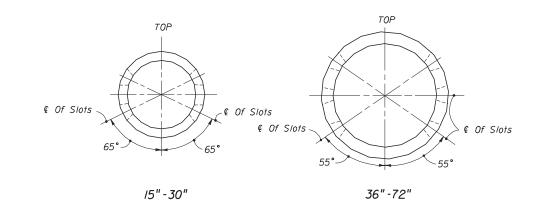
- I. Pipe invert should be at or above the water table whenever possible.
- 2. French drains with minor dimensional changes or otherwise different from the standard cross-section shall be either described or detailed in the plans.

French drains with significantly different cross-sections shall be detailed in the plans.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

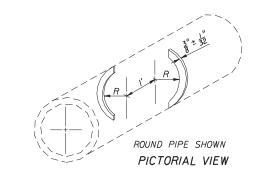
FRENCH DRAIN

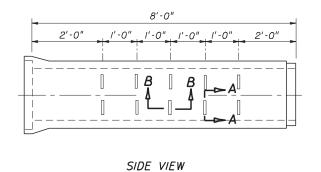
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Drawn By	RWR	09/83	Revision	Sheet No.	Index No.
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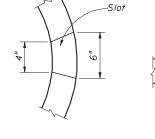


ELLIPTICAL PIPE					
	Slot Cut				
Pipe Size	Opening C				
	Min.	Max.			
14"x23"	10"	12"			
19"x30"	14"	16"			
24"x38"	14"	16"			
29"x45"	20"	22"			
34"x53"	20"	22"			
38"x60"	20"	22"			

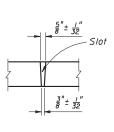
ROUND PIPE						
	Slot Cut					
Pipe Size	Opening C					
	Min.	Max.				
<i>1</i> 5"	12"	14"				
18"	12"	14"				
24"	<i>16"</i>	18"				
30"	16"	18"				
36"	22"	24"				
42"	22"	24"				
48"	22"	24"				
54"	24"	26"				
60"	24"	26"				
66"	24"	26"				
72"	24"	26"				



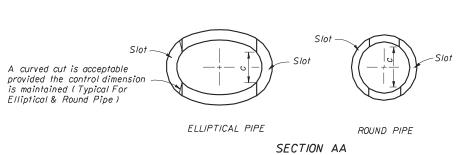


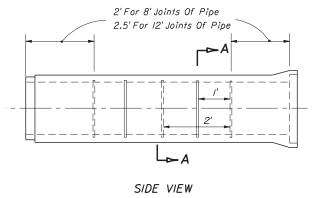


SECTION AA



SECTION BB





OPTION A - ROUND PIPE

OPTION B - ROUND OR ELLIPTICAL PIPE

SLOTTED PIPE OPTIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

FRENCH DRAIN

	Names	Dates	Approve	d By	1 0	
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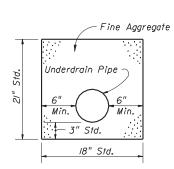
GENERAL NOTES

- I. The underdrain pipe shall be either 4" smooth or 5" corrugated tubing unless otherwise shown in the plans. The size to be furnished will be based on the nominal internal diameter of a pipe with a smooth interior wall. Except when prohibited by the plans, the special provisions or this standard, pipe with a corrugated interior wall may be provided based on the following size equivalency.
 - 4" smooth interior equivalent to 5" corrugated interior
 - 5" smooth interior equivalent to 6" corrugated interior
 - 6" smooth interior equivalent to 8" corrugated interior
 - 8" smooth interior equivalent to 10" corrugated interior
- 2. Fine aggregate shall be quartz sand meeting the requirements of Sections 902-4 of the Standard Specifications.
- 3. Coarse aggregate shall be gravel or stone meeting the requirements of Sections 901-2 or 901-3.

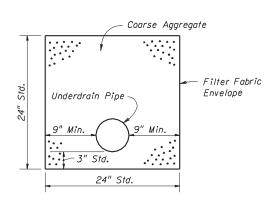
 The gradation shall meet Section 901, Grades 4, 467, 5, 56 or 57 stone unless otherwise shown restricted in the plans.
- 4. Underdrain Type I, II, III and I shall be in accordance with Section 440.
- 5. Filter fabric shall be Type D-3 (See Index No. 199). The internal filter fabric of Type \overline{Y} underdrain shall have a permittivity of 0.7 /sec and an AOS of $^{\#}$ 40 sieve.
- 6. When corrugated polyethylene tubing with slots or 360° perforations is used in conjunction with fine aggregate, a filter fabric sock meeting Section 948 is required.
- 7. See Index no. 500 for the standard location of Type I, II, and III underdrain. The location of Type V underdrain and non standard locations of Type I, II, and III underdrain will be as detailed in the plans.
- 8. All Filter fabric joints shall overlap a minimum of I'. The internal filter fabric of Type \(\mathbb{Y}\) underdrain shall overlap into the coarse aggregate or the fine aggregate a minimum of I'.
- 9. Underdrain outlet pipes shall be non-perforated and all bends shall be made using 1/8 (45 deg.) elbows. 90 deg. bends shall be constructed with two g elbows separated by at least l'of straight pipe. Outlet pipes stubbed into inlets or other drainage structures shall be not less than 6" above the structure flow line. Outlet pipes discharging to grassed areas shall have concrete aprons, hardware cloth, and bordering sod as shown in Index no. 287 for Edgedrain outlets.
- 10. Pay Item shall be based on the size of the smooth interior products. The contract unit price for Underdrain, LF, shall include the cost of pipe, fittings, aggregate, sock, filter fabric, underdrain cleanouts, and concrete parents.

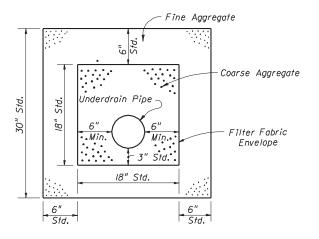
The contract unit price for Underdrain Outlet Pipe, LF, shall be full compensation for trench excavation, pipe and fittings, concrete approns, hardware cloth for concrete aprons, stubbing into drainage structures, backfill in place, and disposal of excess materials.

The contract unit price for Underdrain Inspection Box, EA. shall be for the number completed and accepted.



TYPE I





TYPE II

TYPE 🎹

DESIGN NOTES

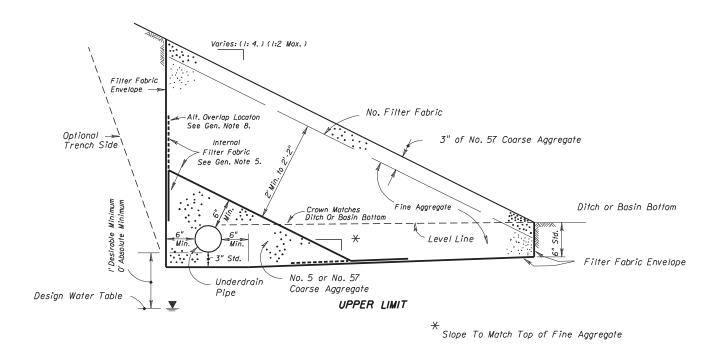
- The type of underdrain should be selected to meet design water removal rate and soil conditions.
 Caution is prescribed in the use of these typical sections since special designs may be required to satisfy project conditions.
- 2. Type I underdrain is intended for minimum water removal conditions.
- Type II underdrain is intended for moderate water removal conditions. Where reactive conditions
 may create chemical clogging, the use of an inert material and/or elimination of the filter fabric
 may be necessary.
- 4. Type III underdrain is intended for maximum water removal conditions. Filter fabric is required between the coarse aggregate or fine aggregate including those described in general notes 2 and 3. Design note 3 applies for reactive conditions.
- 5. Type I underdrain is intended for use in detention basins and other locations which require a filtration system. The standard fine aggregate specified for Type I underdrain conforms to filtration gradation requirements of Chapter 62-25 F.A.C..
- 6. The designer should detail in the plans, the location of:
 (a) Type

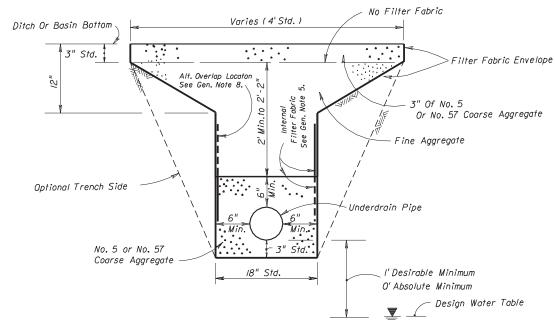
 ✓ underdrain, (b) non-standard locations of Type I, II, and III underdrain, (c)
 underdrain inspection boxes, (d) cleanouts for Type

 ✓ underdrain, and (e) underdrain outlet pipes.
- 7. The designer should specify the flow line elevations at the beginning, bends, junctions and ends of underdrain pipes and outlet pipes.
- 8. The designer should evaluate whether an external filter fabric envelope is required around underdrain Types I and III. When required, fabric shall be specified in the plans.

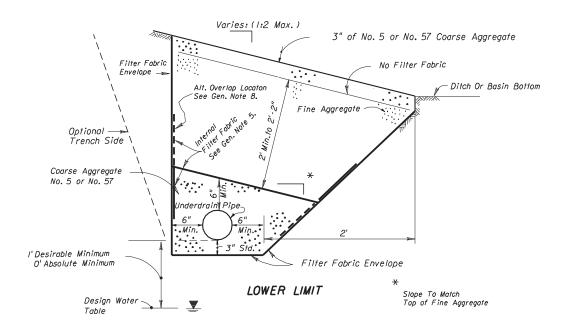
UNDERDRAIN

	Names	Dates	Approve	d By	10
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Drawn By	HSD	10/85	Revision	Sheet No.	Index No.
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TYPE 🗸 b



TYPE **V** a

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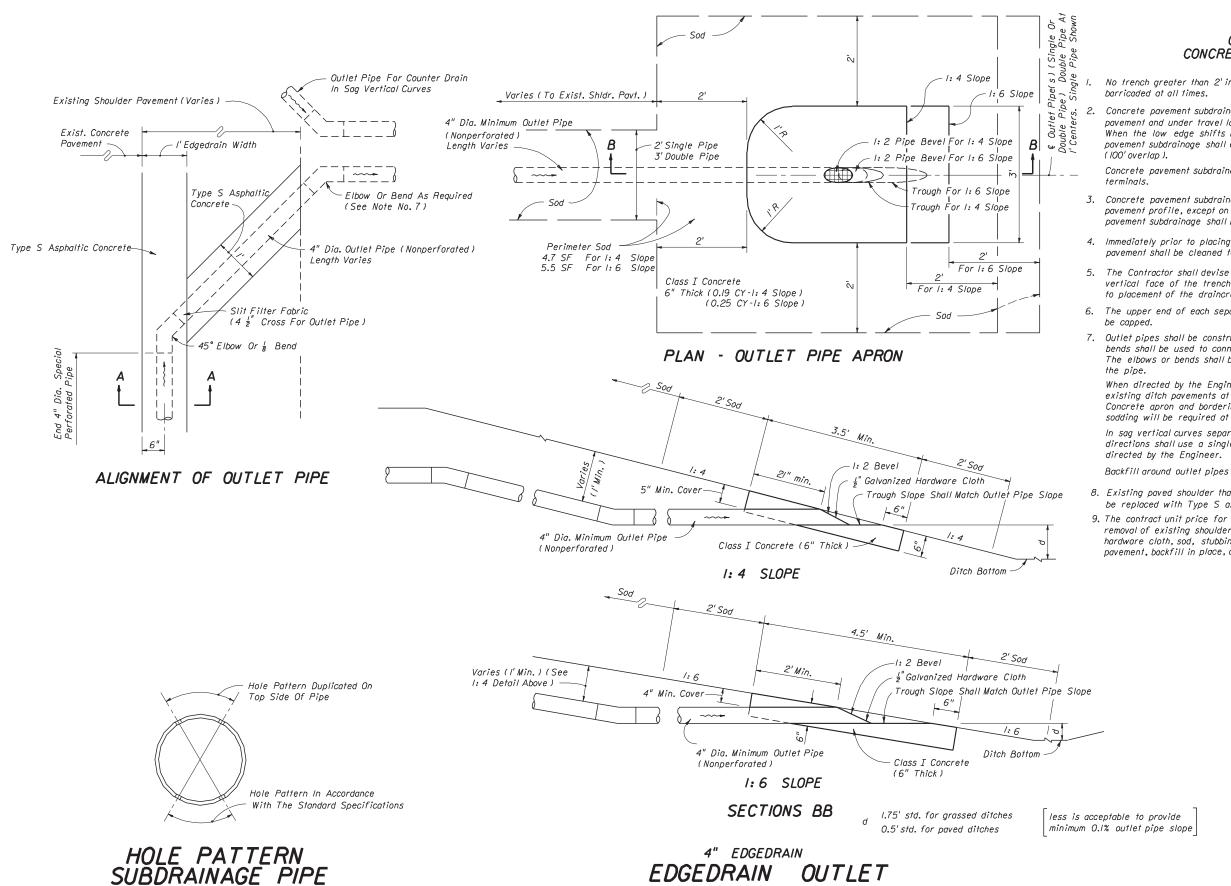
A⊸¬ Warp Apron Surface Threaded Cap Fine Aggregate Conc. Apron Filter Fabric 6" Min. Cleanout Riser Wrap (Non-Perforated) 6" Min. _ · Coarse . Wye Fitting * Aggregate - UnderDrain SECTION AA

CLEANOUT FOR TYPE Y UNDERDRAIN

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

UNDERDRAIN

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GENERAL NOTES FOR CONCRETE PAVEMENT SUBDRAINAGE

- No trench greater than 2' in depth will be allowed overnight. Trenches shall be barricaded at all times.
- 2. Concrete pavement subdrainage shall be constructed adjacent to the low edge of the roadway pavement and under travel lanes, auxiliary pavement and shoulders, as called for in the plans. When the low edge shifts between outside and inside edges of pavement the concrete pavement subdrainage shall extend 50' beyond and begin 50' before the flat point (100' proctor).

Concrete pavement subdrainage shall be placed on the low side of ramps of crossroad terminals.

- 3. Concrete pavement subdrainage shall be constructed on a grade parallel with the edge of pavement profile, except on profiles flatter than one-tenth percent (0.10%) the concrete pavement subdrainage shall be constructed on a grade of one-tenth percent (0.10%).
- 4. Immediately prior to placing the filter fabric the entire vertical face of the concrete pavement shall be cleaned to remove adhering base material and soil.
- The Contractor shall devise a procedure for holding the filter fabric in position on the vertical face of the trench. The procedure must be approved by the Engineer prior to placement of the draincrete.
- The upper end of each separate run of the concrete pavement subdrainage pipe shall be capped.
- 7. Outlet pipes shall be constructed at a maximum of 500' intervals. Elbows or $\frac{1}{8}$ bends shall be used to connect the outlet pipe to the concrete pavement subdrain pipe. The elbows or bends shall be of the same material as the outlet pipe but compatible with the pipe.

When directed by the Engineer, outlet pipes shall be stubbed into existing inlets or into existing ditch povements at an elevation 6" above the inlet flowline or ditch bottom. Concrete apron and bordering sod are not required for stubbed outlets, but replacement sodding will be required at trenches for pipes stubbed into paved ditches.

In sag vertical curves separate outlet pipes for concrete pavement subdrains from opposite directions shall use a single apron unless otherwise shown in the plans or otherwise directed by the Engineer.

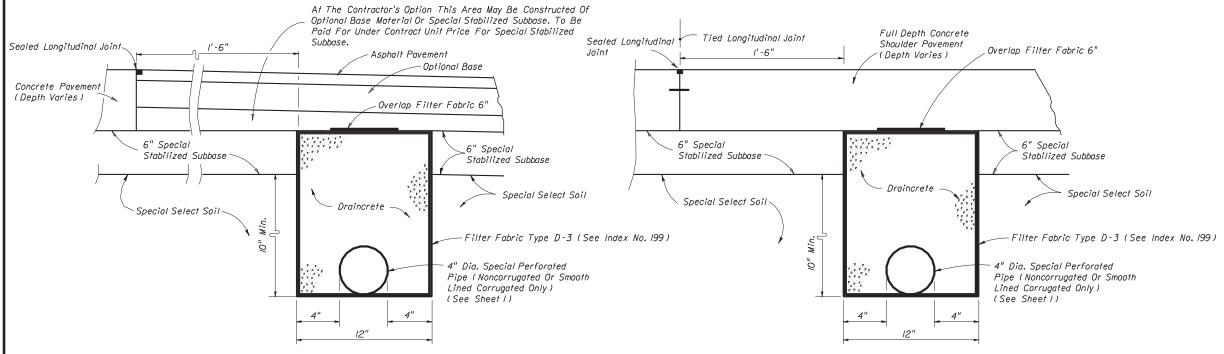
Backfill around outlet pipes shall be of cohesive soils, draincrete will not be permitted.

- 8. Existing paved shoulder that is removed for the construction of outlet pipes shall be replaced with Type S asphaltic concrete at the rate of 500 LB per SY.
- 9. The contract unit price for Edgedrain Outlet Pipe (4") LF, shall be full compensation for removal of existing shoulder pavement, trench excavation, pipe and fittings, concrete apron, hardware cloth, sod, stubbing into existing inlets and paved ditches, restoration of ditch pavement, backfill in place, and disposal of excess materials.

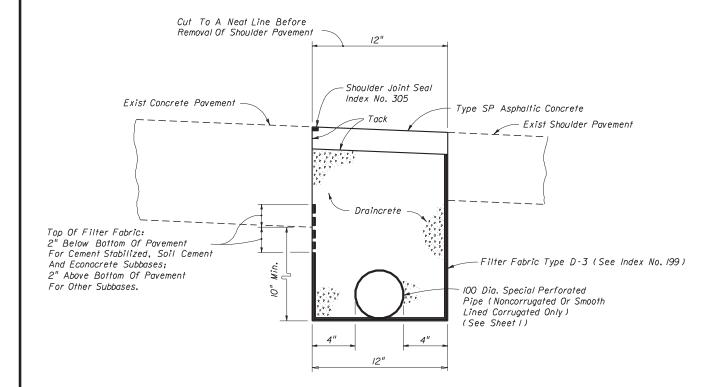
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

CONCRETE PAVEMENT SUBDRAINAGE

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Drawn By	DLD	10/94	Revision	Sheet No.	Index No.	
Checked By	HMD/WPH	10/94	00	l of 3	287	



NEW CONSTRUCTION



REHABILITATION

ASPHALT SHOULDERS

DRAINCRETE SUBDRAINAGE

CONCRETE TRAVEL LANES,

SHOULDERS. AND AUXILIARY PAVEMENT

- NOTES FOR DRAINCRETE PAVEMENT SUBDRAINAGE
- I. The edgedrain sections for DRAINCRETE SUBDRAINAGE are appliable to pavement construction identified as RIGID PAVEMENT Alteration #I on Index No. 505 (sheet 2 of 3)
- The contractor shall confine the construction of draincrete edgedrain to an area in which
 the entire operation can be carried out in five (5) work days, unless another construction
 period is called for in the plans, with sufficient time allowed for the draincrete to set
 before placement of pavement.

METHOD OF PAYMENT

NEW CONSTRUCTION:

 The contract unit price for Edgedrain (Draincrete) LF shall be full compensation for trench excavation disposal of excess material, filter fabric, draincrete edgedrain pipe and fittings and draincrete.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet I of 3.

FOR REHABILITATION:

I. The contract unit price for Edgedrain (Draincrete) LF, shall be full compensation for removal of existing shoulder pavement, trench excavation, disposal of excess materials, filter fabric, draincrete edgedrain pipe and fittings, draincrete, and barricades necessary for edgedrain construction.

Payment for outlet pipe shall be in accordance with General Note 9, Sheet I of 3.

Shoulder payement shall be paid for under the contract unit price for Type SP, Asphatic

Tack coat shall be paid for under the contract unit price for Bit Matl (Tack Coat), GA.

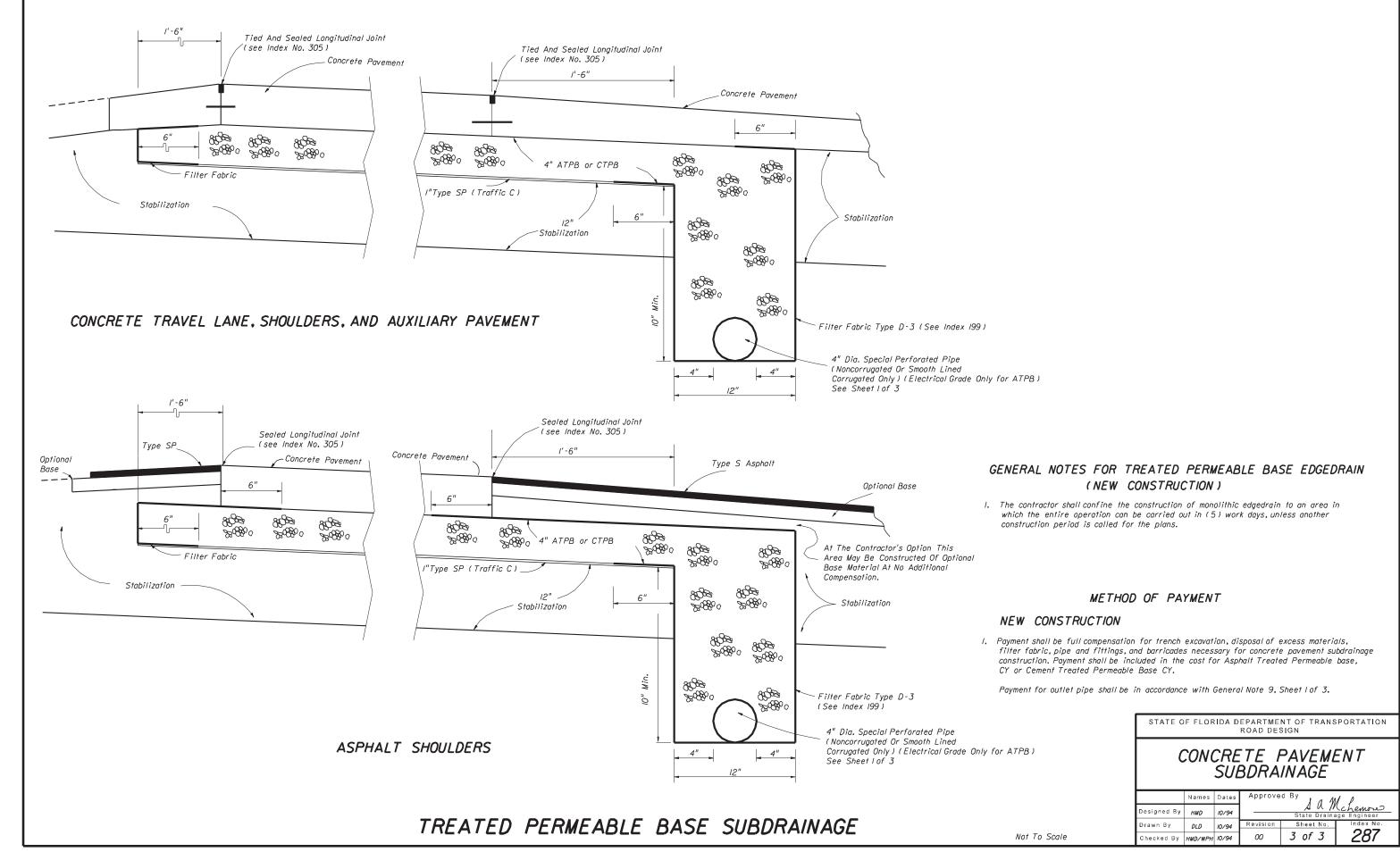
Shoulder joint seal shall be paid for under the contract unit price for Pavement joint or, LF.

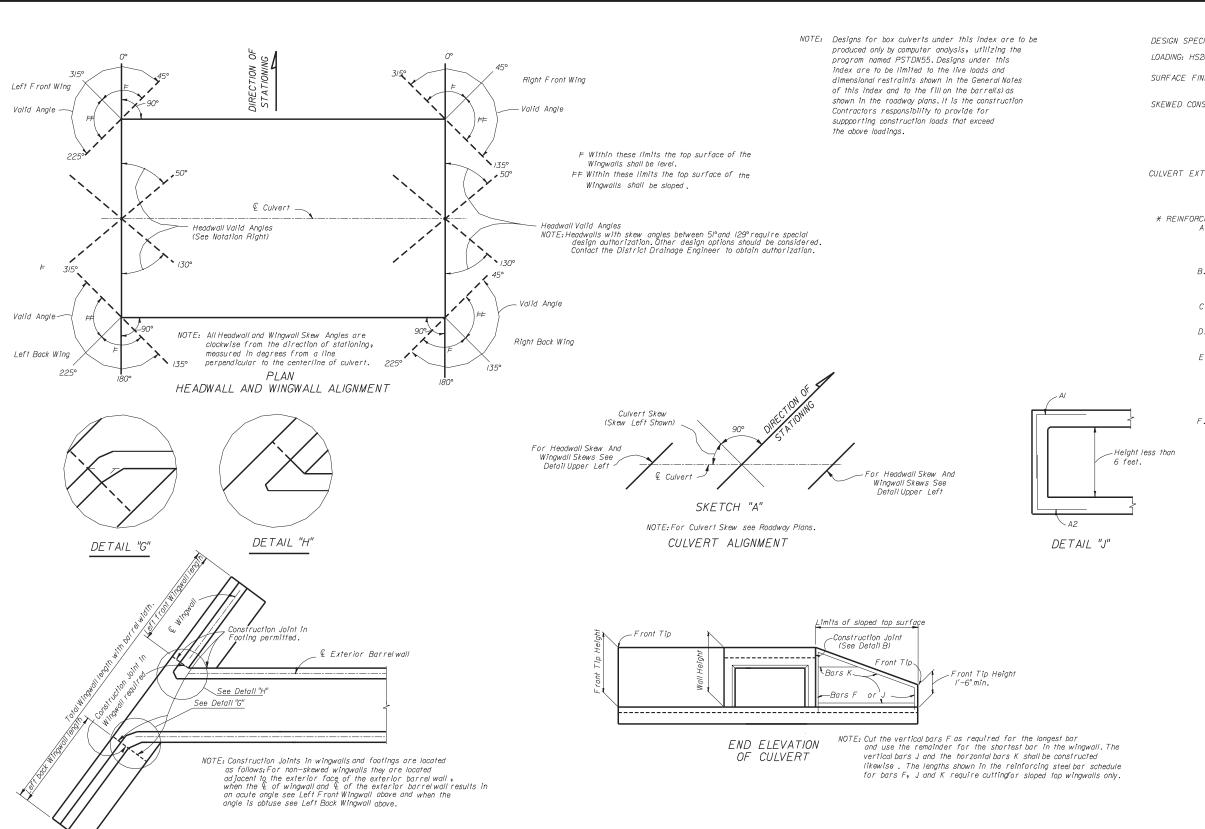
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CONCRETE PAVEMENT SUBDRAINAGE

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ı	Designed By	HMD	10/94	State Drainage Engineer			
	Drawn By	DLD	10/94	Revision	Sheet No.	Index No.	
ı	Checked By	HMD/WPH	10/94	00	2 of 3	287	

Not to scale





PART PLAN SHOWING WINGWALLS AND THE LOCATION
OF CONSTRUCTION JOINTS

GENERAL NOTES

DESIGN SPECIFICATIONS: A.A.S.H.T.O. 1996.

LOADING: HS20-44, Modified for Military Loading as Required or HS25, see Structures Design Guidelines.

SURFACE FINISH: The Class Surface finish for all concrete surfaces shall be a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, in the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with the table (lower right) this sheet. The cost of construction joints shall be at the expense of the contractor.

CULVERT EXTENTIONS: For cut backs and ties into existing concrete box culverts see Index No. 280

* REINFORCING BAR SCHEDULE:

- A. When the depth is less than or equal to 2.0 feet, Bars C2 are utilized in the bottom of the top slab. In all other cases, Bars C2 are replaced with Bars CI spaced at 18 Inches on centers.
- B. When the skew angle for a headwall equals O degrees plus or minus II degrees the respective S Bars (S2 or S3) will not be utilized.
- C. When the barrel height is less than 6 Feet, Bars B2 will be eliminated as shown in Detail J.
- D. If the span is less than five feet, Bars Al and A2 will be Type II Bars.
- E. The portions of Bars "N" that extend thru Construction Joints into wingwalls above footings shall be given one coat of approved zinc rich point and shall be encased in approved capped plastic (PVC) pipes filled with approved durable lubricant or cut back asphalt. The length and inside diameter of the plastic pipe shall be approximately \(\frac{1}{2} \)" larger than those of the bar.
- F. For culvert extensions Bar Cl is redesignated Bar C3 in the bottom slab.

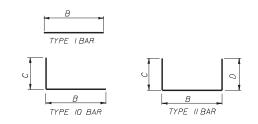


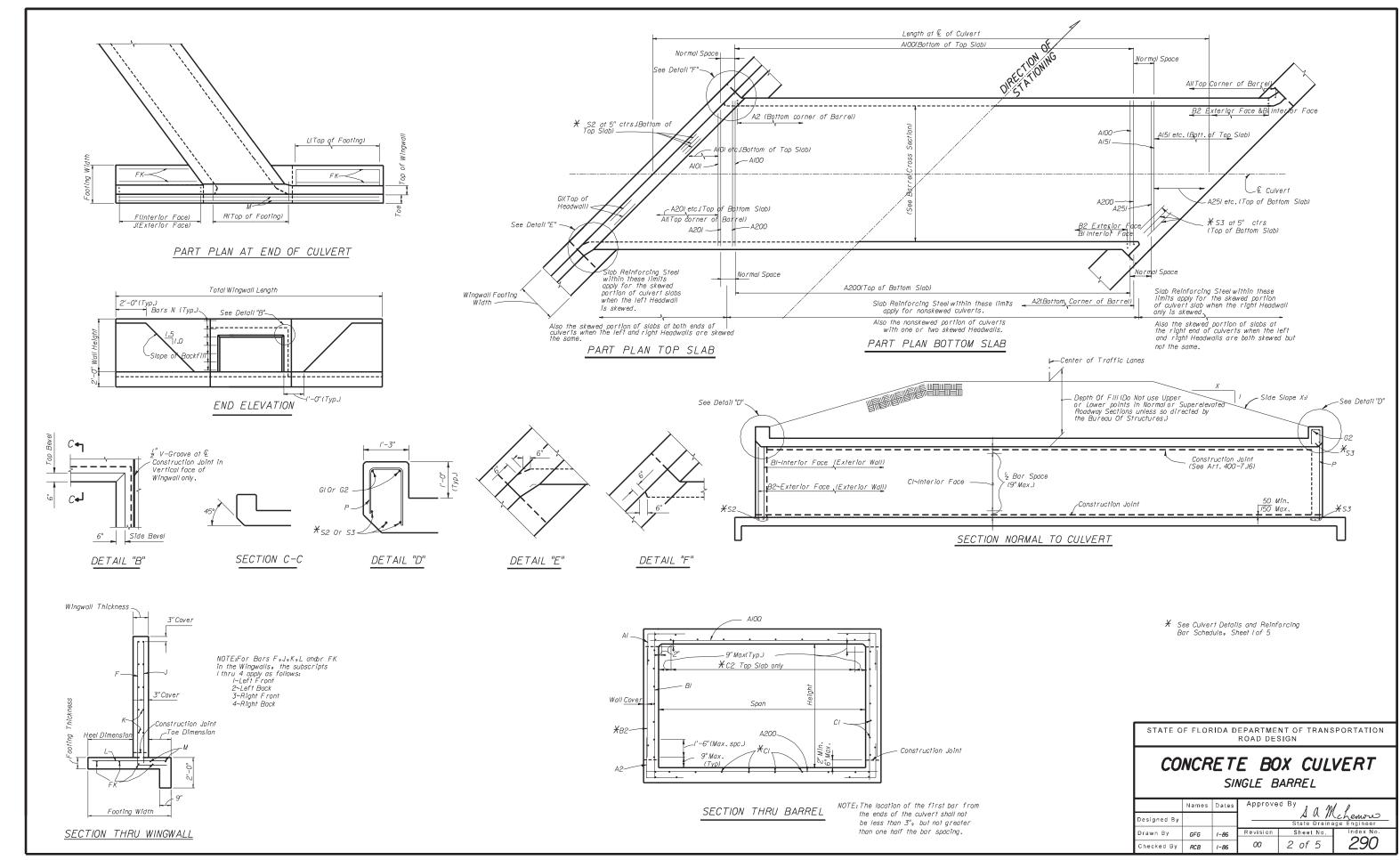
TABLE OF MINIMUM BAR SPLICE LENGTHS

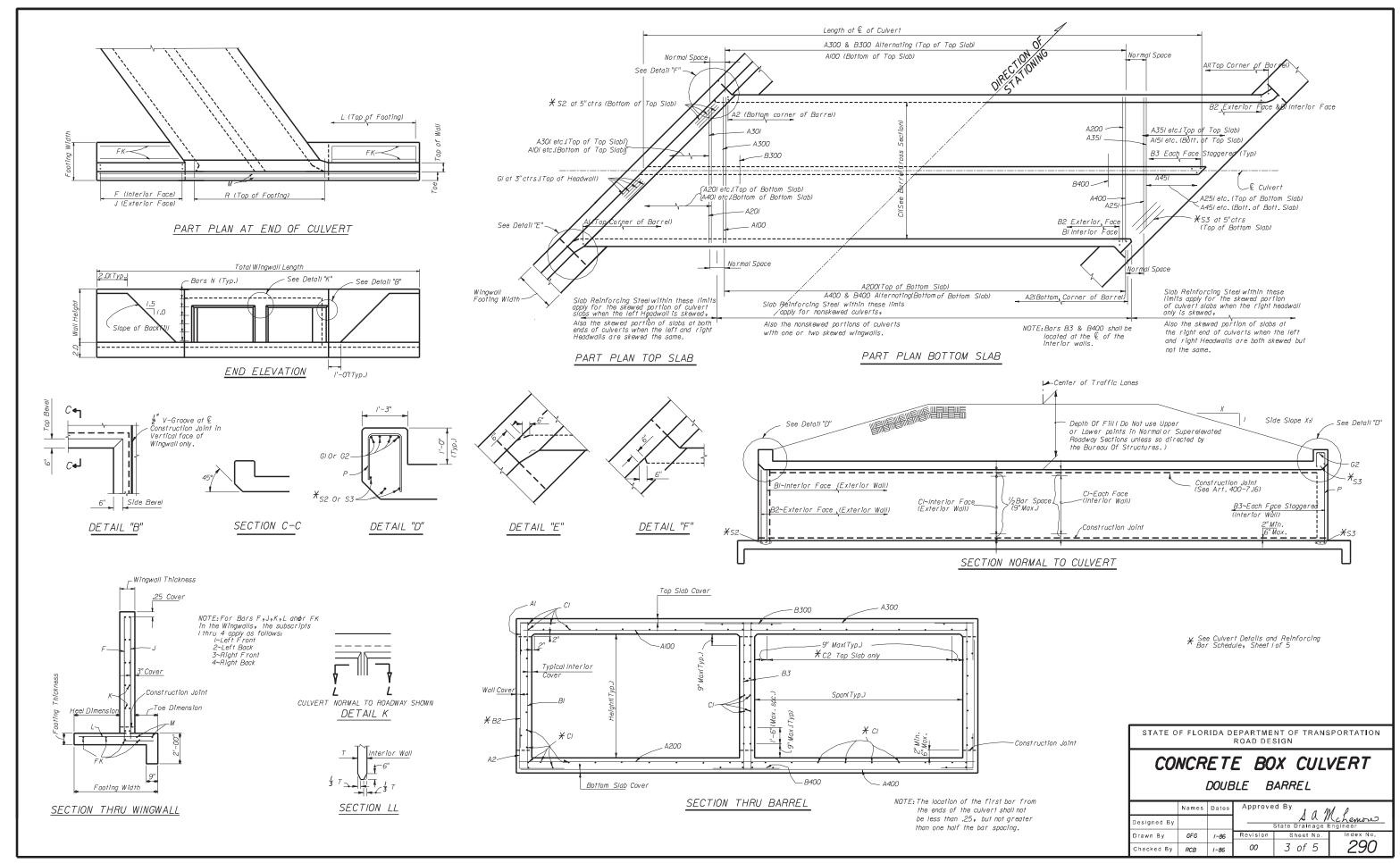
BAR SIZE	SPLICE	BAR SIZE	SPLICE
#4	/'-/0"	#8	4'-8"
#5	2'-4"	#9	5'-3"
#6	2'-9"	#/0	5'-10"
#7	⊿ ′-∩"	#//	6'-6"

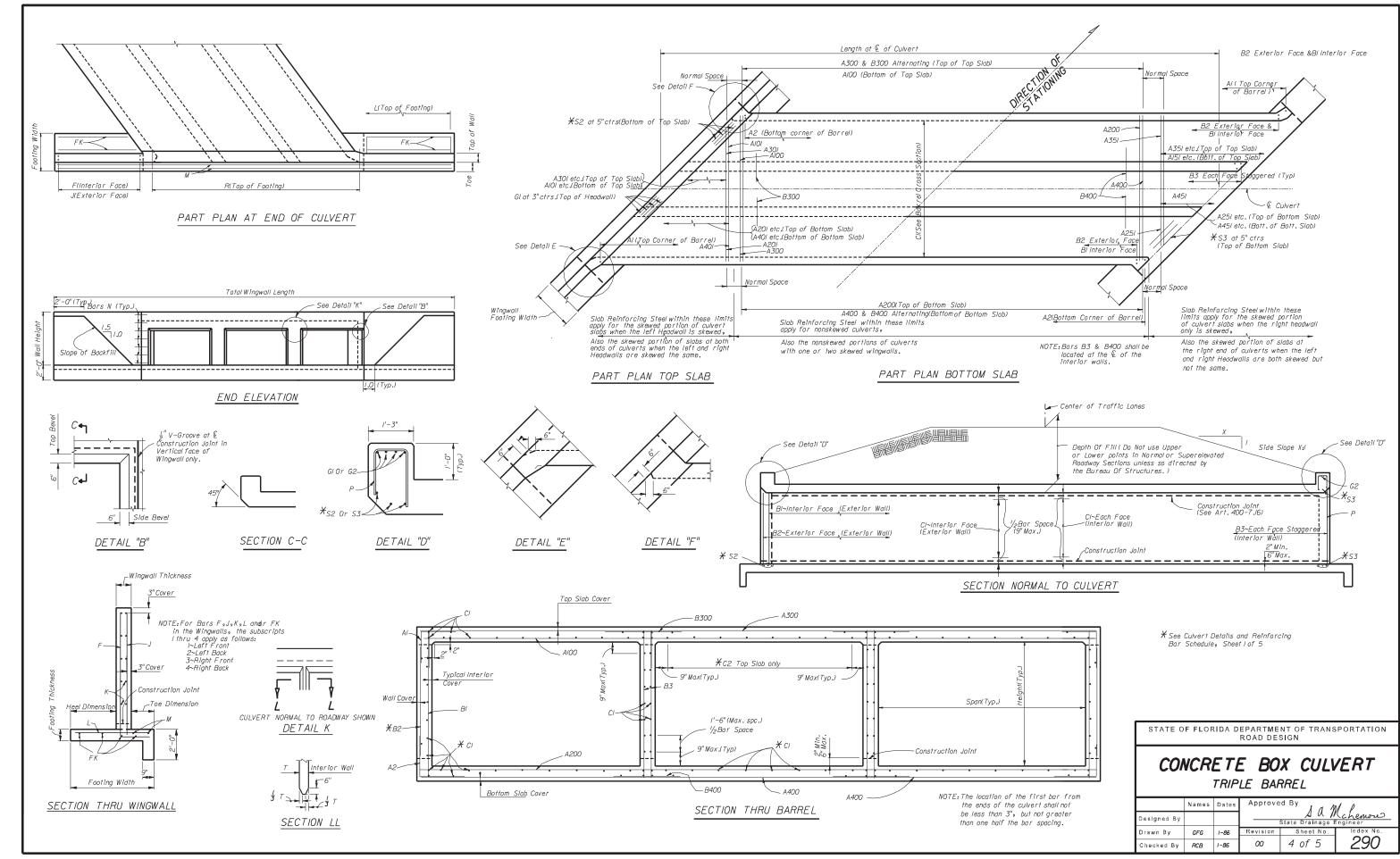
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

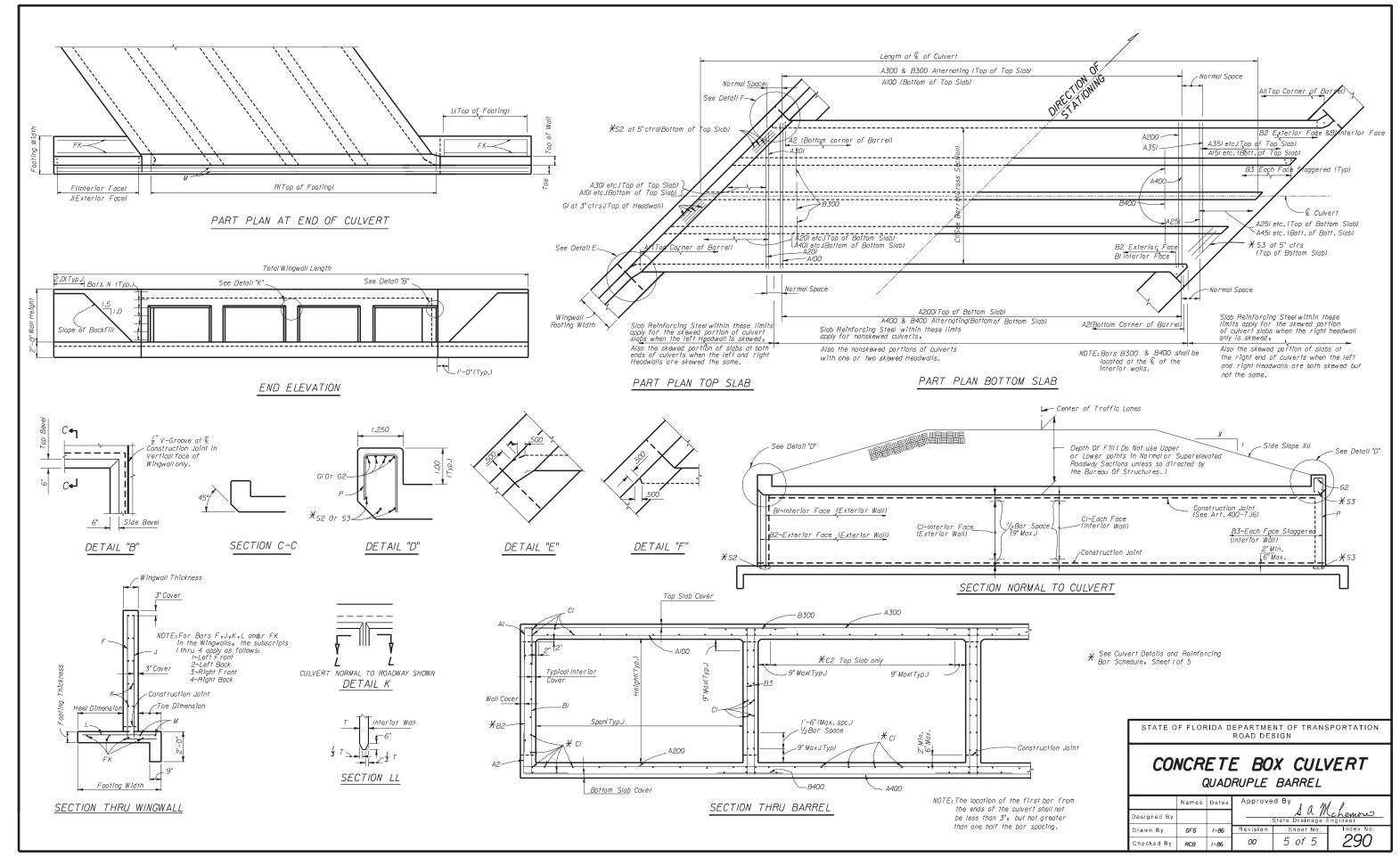
CONCRETE BOX CULVERT CULVERT DETAILS

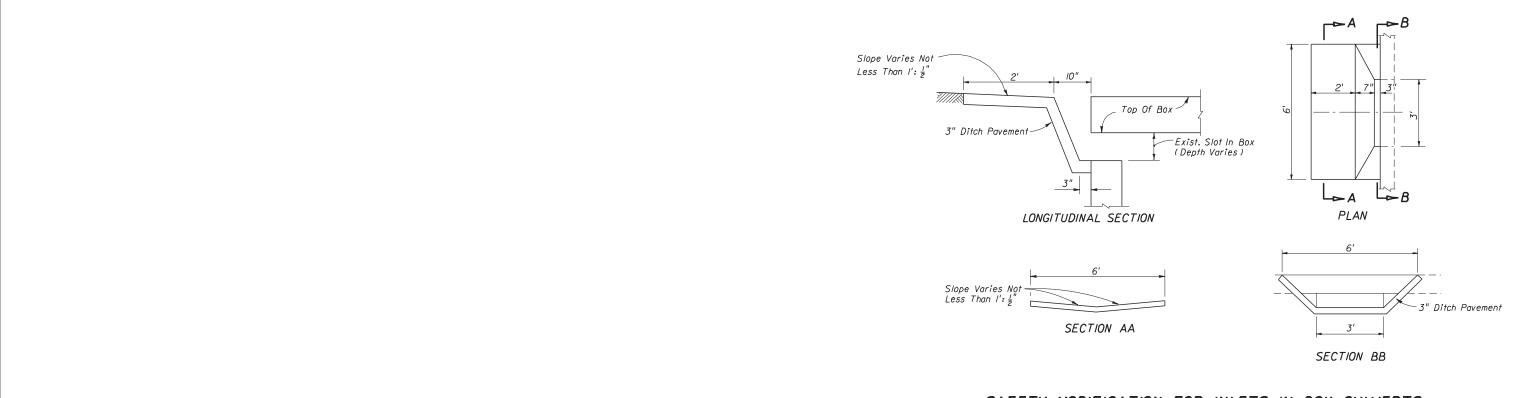
	Names	Dates	Approve	1/1	M/P		
Designed By			State Drainage Engineer				
Drawn By	GFG	1-86	Revision	Sheet No.	Index No.		
Checked By	RCB	I-86	00	1 of 5	290		









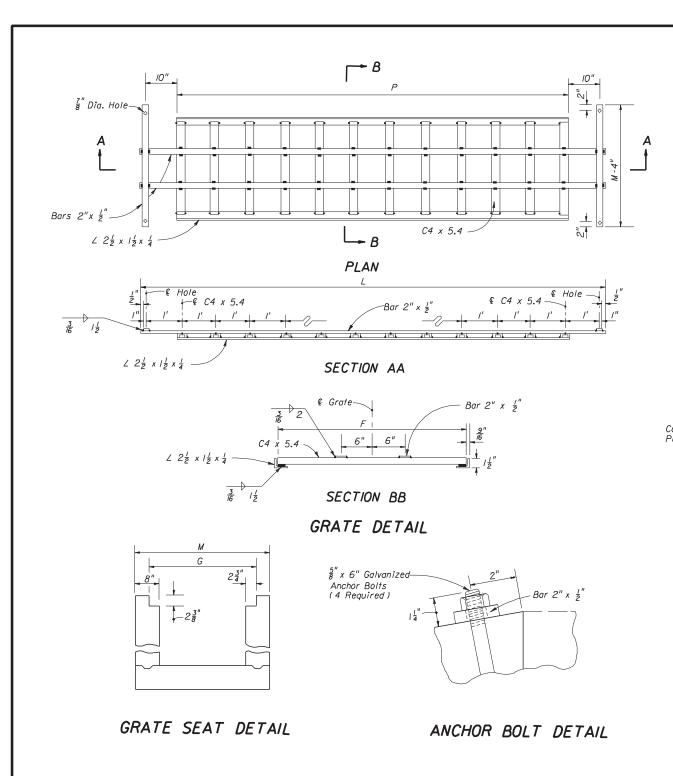


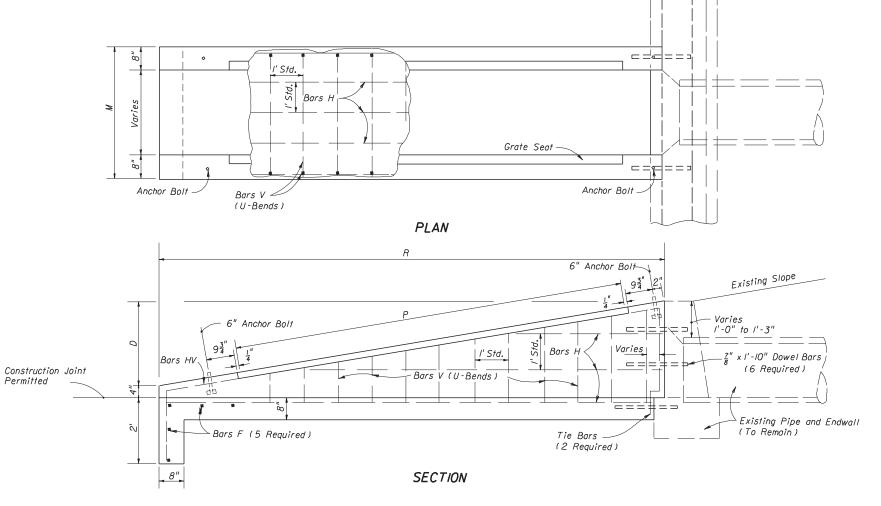
SAFETY MODIFICATION FOR INLETS IN BOX CULVERTS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SAFETY MODIFICATIONS FOR INLETS IN BOX CULVERTS

	Names	Dates	Approve	d By	M P
Designed By	HAB	07/67		(chemorio	
Drawn By	MJT	07/67	Revision	Sheet No.	Index No.
Checked By	DWS	07/67	00	I of I	293





- I. For use criteria see " Steel Grating Use Criteria " Index No. 261.
- 2. Grates shall be ASTM A242/A242M, A572/A572M or A588/A588M, Grade 50 steel, and galvanized in accordance with Section 962-7 of the Standard Specifications.
- 3. Channel section C3 x 6.0 may be substituted for the C4 x 5.4 channel.
- 4. All reinforcing No. 4 bars with 2" clearance except as noted. Spacings shown are center to center. Laps to be 12" minimum. Welded wire fabric (two cages max.) having an equivalent cross section area (0.20 sq. in.) may be substituted for bar reinforcement.
- 5. Drill 13" holes 8" deep with a rotary drill in existing endwall for dowel bars. Holes shall be thoroughly cleaned prior to placing dowel bars and epoxy.

GENERAL NOTES

- 6. Endwall to be paid for under the contract unit price for Class I Concrete (Endwalls), CY and Reinforcing Steel (Roadway), LB. Cost of dowel bars and epoxy mortar to be included in the contract unit price for reinforcing steel. Cost of grates to be paid for under the contract unit price for Endwall Grate, LB., plan quantity. Cost of galvanized bolts and nuts to be included in the contract unit price for the grate.
- 7. Sod slopes 5' each side and above endwall. Sodding to be paid for under the contract unit price for Sodding, SY.

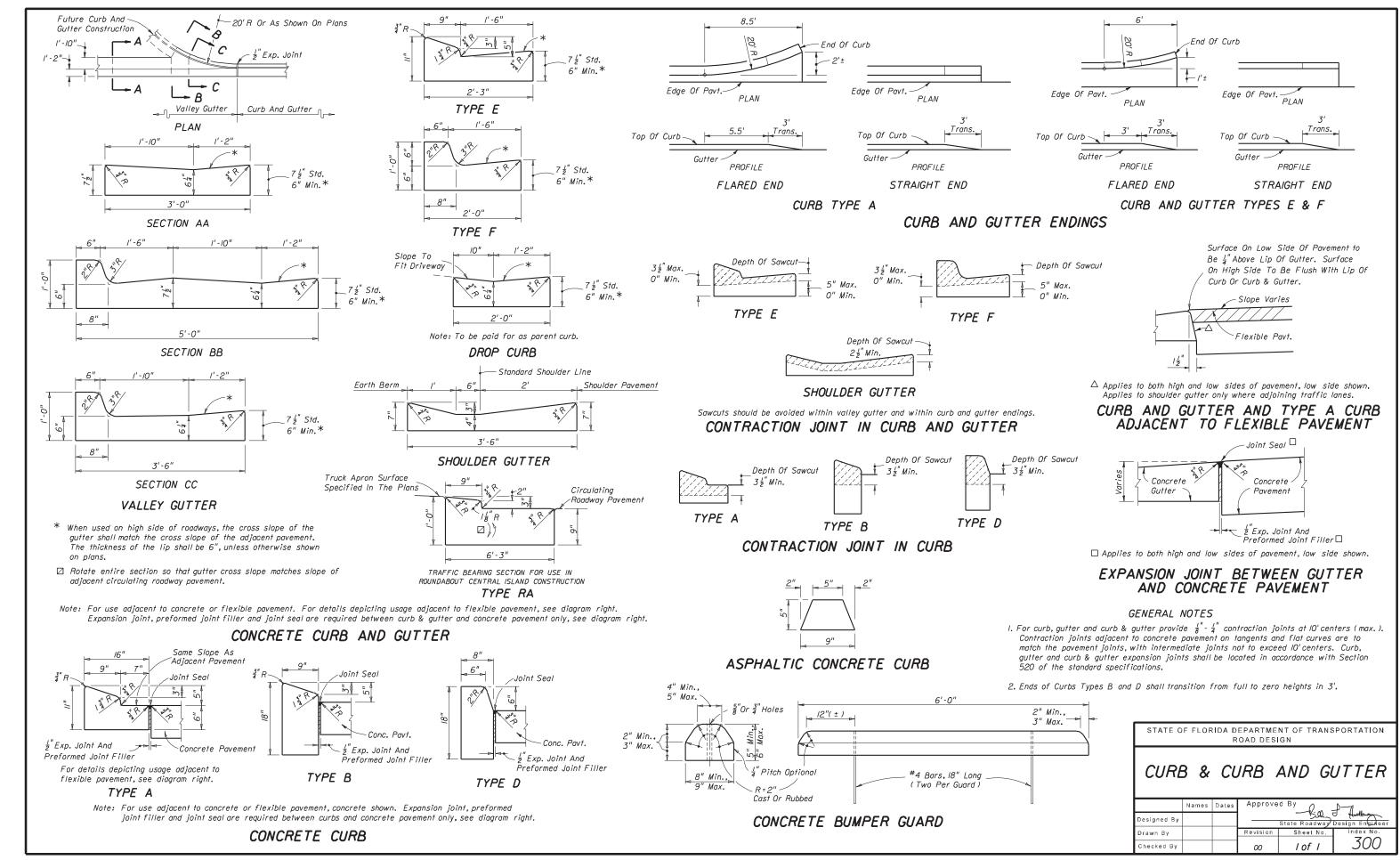
DIMENSIONS AND QUANTITIES PER GRATE										
Clana	Pipe	Channels	@ 5.4 Lbs	5./L.F.	Bars @ .	3.4 <i>lbs/L.F</i> .	(2 ea.)	Angles @ 3.2	Lbs./L.F. (2)	Total
Slope	Size	Quantity	F	Lbs.	L	M-4"	Lbs.	P	Lbs.	Weight-Lbs.
	<i>1</i> 5"	10	2'-6 7 "	/39	//'-3"	3'-3"	99	9'-4"	60	298
1 : 6	18"	12	2'-9 7 "	183	13'-3"	3'-6"	114	11'-4"	73	370
7.0	24"	15	3'-3 7 "	269	16'-3"	4'-0"	/38	14'-4"	92	499
	30"	18	3'-9 7 "	372	19' - 3"	4'-6"	162	17'-4"	III	645
	15"	6	2'-67"	83	7'-3"	3'-3"	7/	5'-4"	.34	188
, ,	18"	7	2'-97"	107	8'-3"	3'-6"	80	6'-4"	41	228
1:4	24"	9	3'-3 ⁷	161	10'-3"	4'-0"	97	8'-4"	53	311
	30"	11	3'-9 7 "	227	12'-3"	4'-6"	114	10'-4"	66	407

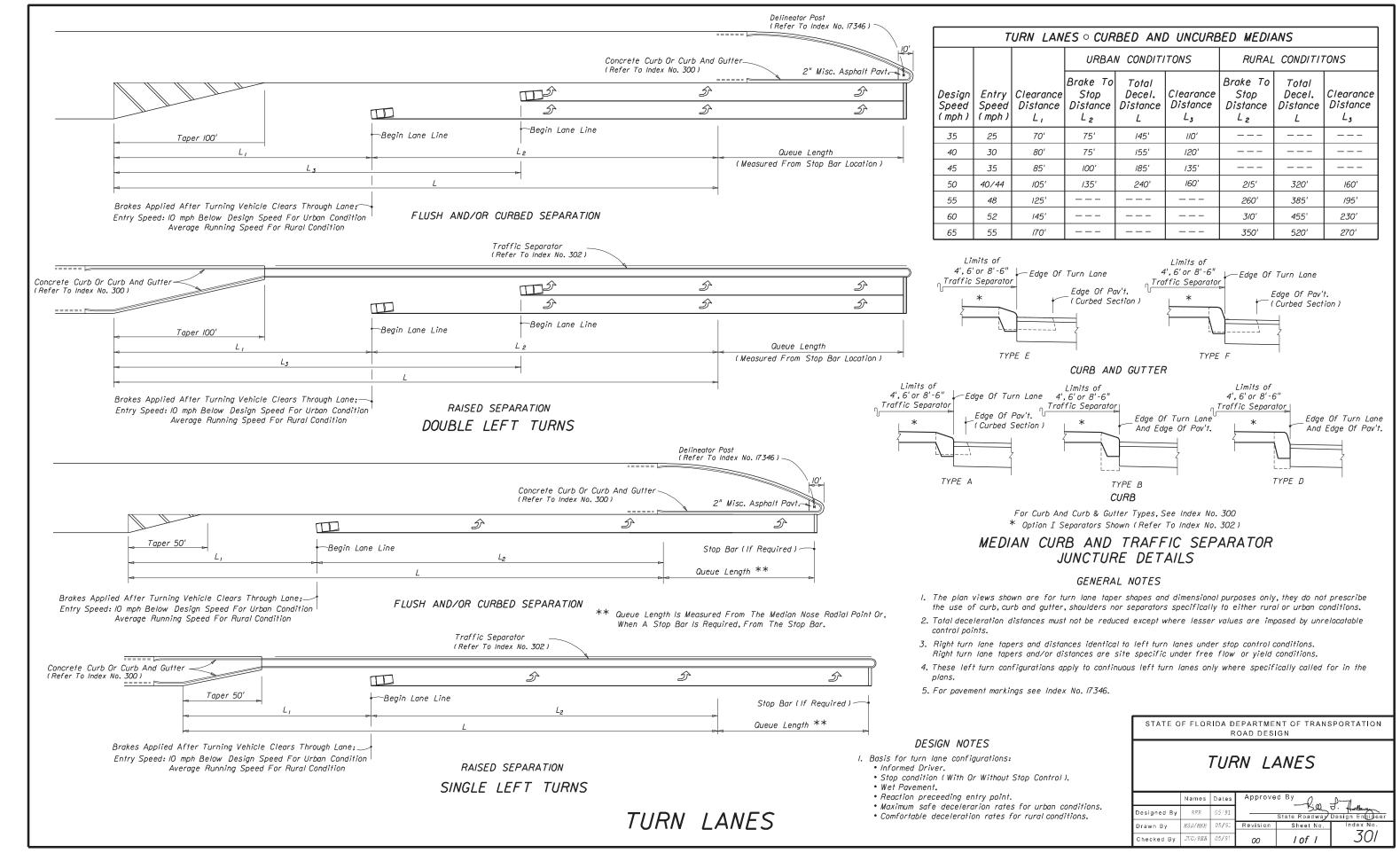
		DII	MENSIO	VS ANL	QUAN	TITIES	PER U-E	NDWALL	
	Pipe Size	G	М	D	R	Р	Class I Concrete-C.Y.	Reinforcing Steel-Lbs.	Sodding S Y
] [<i>15"</i>	2'-8 <u>-</u> 1"	3'-7"	2'-2"	13'-0"	9'-4"	2.12	167	23
1 [18"	2' - ½"	3'-/0"	2'-5"	14' -6"	//'-4"	2.53	173	25
	24"	3'-5 <u> </u> "	4'-4"	2'-11"	17'-6"	14' - 4"	3.48	238	29
	30"	3' - // ½"	4'-10"	3'-5"	20'-6"	17'-4"	4.57	3/5	32
1 [1511	01.01"	3'-7"	2'-2"	01.01			100	10
	15"	2'-8½"	3 -/ "	2-2	8'-8"	5'-4"	1.44	120	19
	18"	2' - 11 ½"	3'-10"	2'-5"	9'-8"	6'-4"	1.72	130	20
	24"	3' -5½"	4'-4"	2'-11"	11'-8"	8'-4"	2.36	167	22
] [30"	3' - // ½"	4'-10"	3'-5"	/3'-8"	10' -4"	3.09	225	25

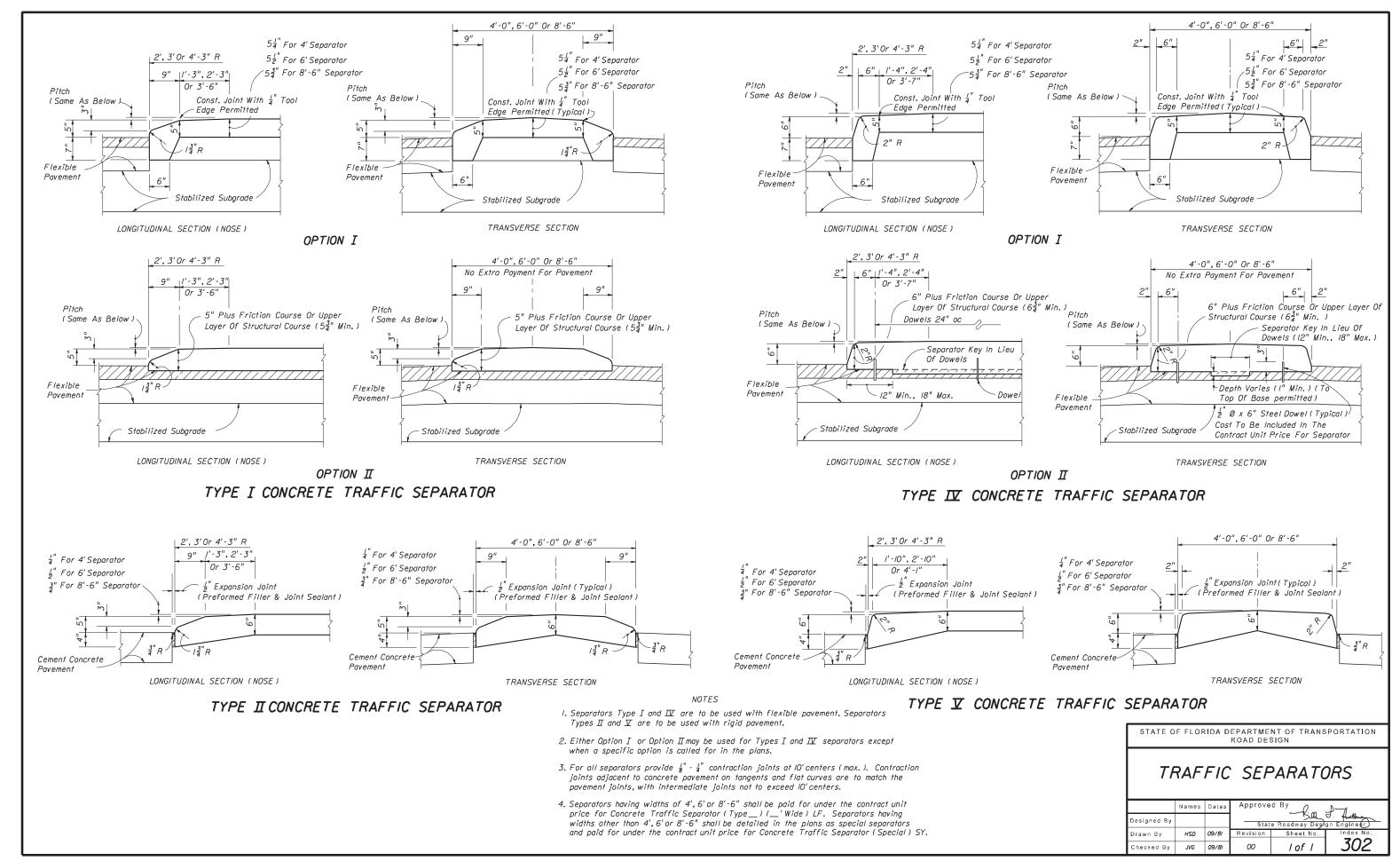
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

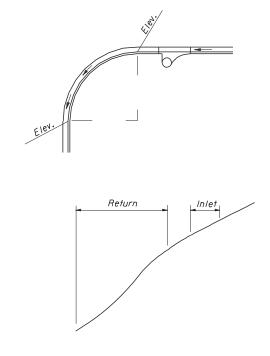
SAFETY MODIFICATIONS FOR ENDWALLS

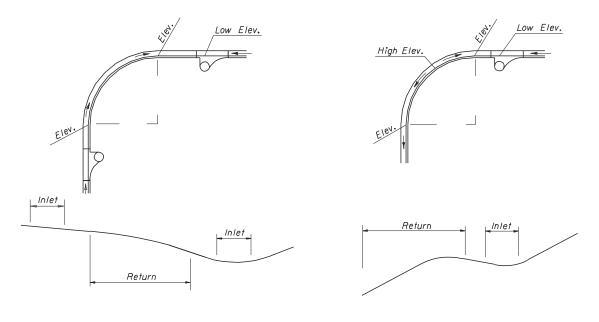
	Names	Dates	Approve	d By	M D			
Designed By			State Drainage Engineer					
Drawn By			Revision	Sheet No.	Index No.			
Checked By			00	l of l	295			

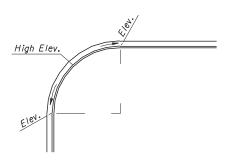


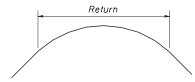












Note:

Profile grades should be established that will allow inlets to be located outside the return whenever practical. Inlets should be located to avoid conflict with pedestrian movement. Special care must be exercised to prevent conflict with public sidewalk curbed ramps for the disabled. For information on public sidewalk curbed ramps refer to Index No. 304.

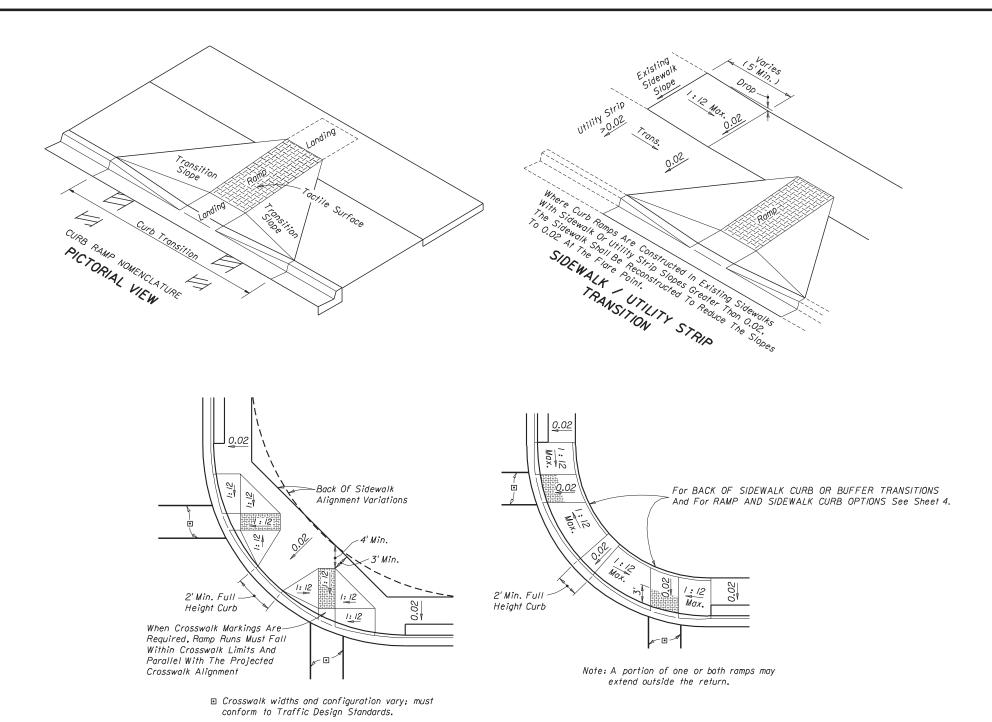
SHOWING LOCATION OF INLETS ON RETURN

TYPICAL RETURN PROFILES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

CURB RETURN PROFILES

	Names	Dates	Approve	d By	♪ ─⁄/
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	l of l	303



TYPICAL PLACEMENT OF PUBLIC SIDEWALK CURB RAMPS AT CURBED RETURNS

GENERAL NOTES

- I. Public sidewalk curb ramps shall be constructed in the public right of way at locations that will provide continuous unobstructed pedestrian circulation paths to pedestrian areas, elements and facilities in the public right of way and to accessible pedestrian routes on adjacent sites. Curbed facilities with sidewalks and those without sidewalks are to have curb ramps constructed at all street intersections and at turnouts that have curbed returns. Partial curb returns shall extend to the limit prescribed by Index No. 515 to accommodate curb ramps. Ramps constructed at locations without sidewalks shall have a landing constructed at the top of each ramp, see Sheet 5.
- 2. The location and orientation of curb ramps shall be as shown in the plans.
- 3. Curb ramp running slopes at unrestrained sites shall not be steeper than I: 12 and cross slope shall be 0.02 or flatter.

When altering existing pedestrian facilities where existing site development precludes the accommodation of a ramp slope of l:l2, a running slope between l:l2 and l:l0 is permitted for a rise of 6" maximum and a running slope of between l:l0 and l:8 is permitted for a rise of 3" maximum. Where compliance with the requirements for cross slope cannot be fully met, the minimum feasible cross slope shall be provided.

Ramp running slope is not required to exceed 8' in length, except at sites where the plans specify a greater length.

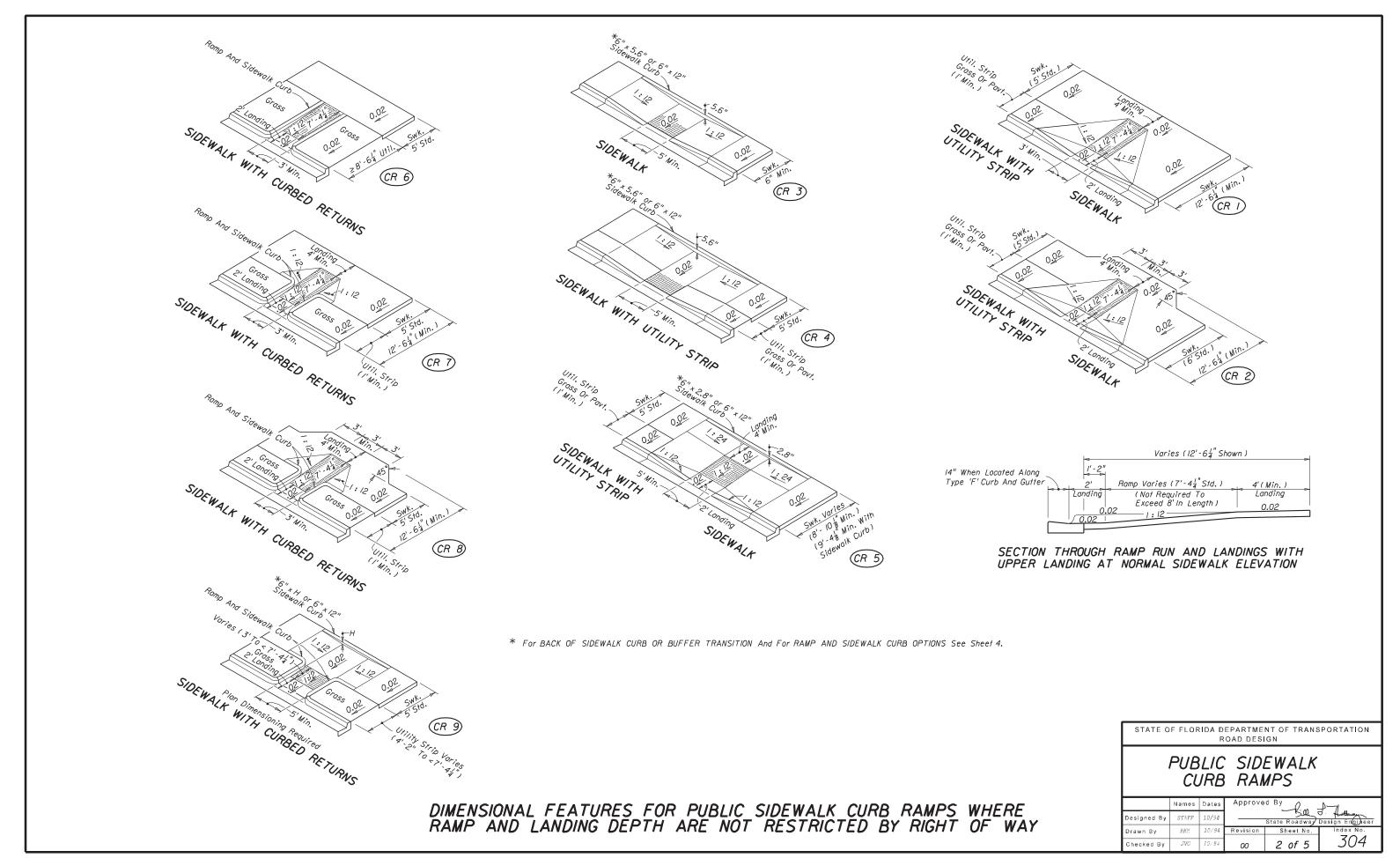
- 4. If a curb ramp is located where pedestrians must walk across the ramp, then the walk shall have transition slopes to the ramp; the maximum slope of the transitions shall be 1:12. Ramps with curb returns may be used at locations where other improvements provide guidance away from that portion of curb perpendicular to the sidewalk; improvements for guidance are not required at curb ramps for linear pedestrian traffic.
- 5. When perpendicular curb ramps abut the back of curb a tactile surface shall be applied to the full width and length of the ramp. When landings of parallel curb ramps abut the back of curb the tactile surface shall extend full width and 3' back of the curb. The tactile surfaces on curb ramps for linear pedestrian traffic and for corner ramps under conditions of infeasibility are to conform with the details in this Index for those specific ramp types. Tactile surfaces shall be constructed by texturing to a depth not exceeding $\frac{1}{8}$ " by use of a tamp or roller fabricated with an imprinting surface of either I" mesh $\frac{1}{4}$ " wire cloth (plain weave, conventional crimp) $\frac{1}{8}$ 6 expanded metal (standard) or 3 lb. expanded metal grating. Transition slopes are not to have tactile surfaces. Detectable surface requirements have been suspended; if reinstated the detectable surface requirements will replace the tactile surface requirements and notice will be by special provision.
- 6. Where a curb ramp is constructed within existing curb, curb and gutter and/or sidewalk, the existing curb or curb and gutter shall be removed to the nearest joint beyond the curb transitions or to the extent that no remaining section of curb or curb and gutter is less than 5' long. The existing sidewalk shall be removed to the nearest joint beyond the transition slope or walk around or to the extent that no remaining section of sidewalk is less than 5' long.
- 7. Alpha-numeric identifications are for reference (plans, permits, etc.).
- 8. Public sidewalk curb ramps are to be paid for as follows:
 Ramps, reconstructed sidewalks, walk around sidewalks, sidewalk landings and sidewalk
 curbs are to be paid for under the contract unit price for Sidewalk Concrete, (__" Thick),
 SY. Curb transitions and reconstructed curbs are to be paid for under the contract
 unit price for the parent curb, i.e., Curb Conc., (Type __), LF or Curb and Gutter Conc.,
 (Type __), LF.

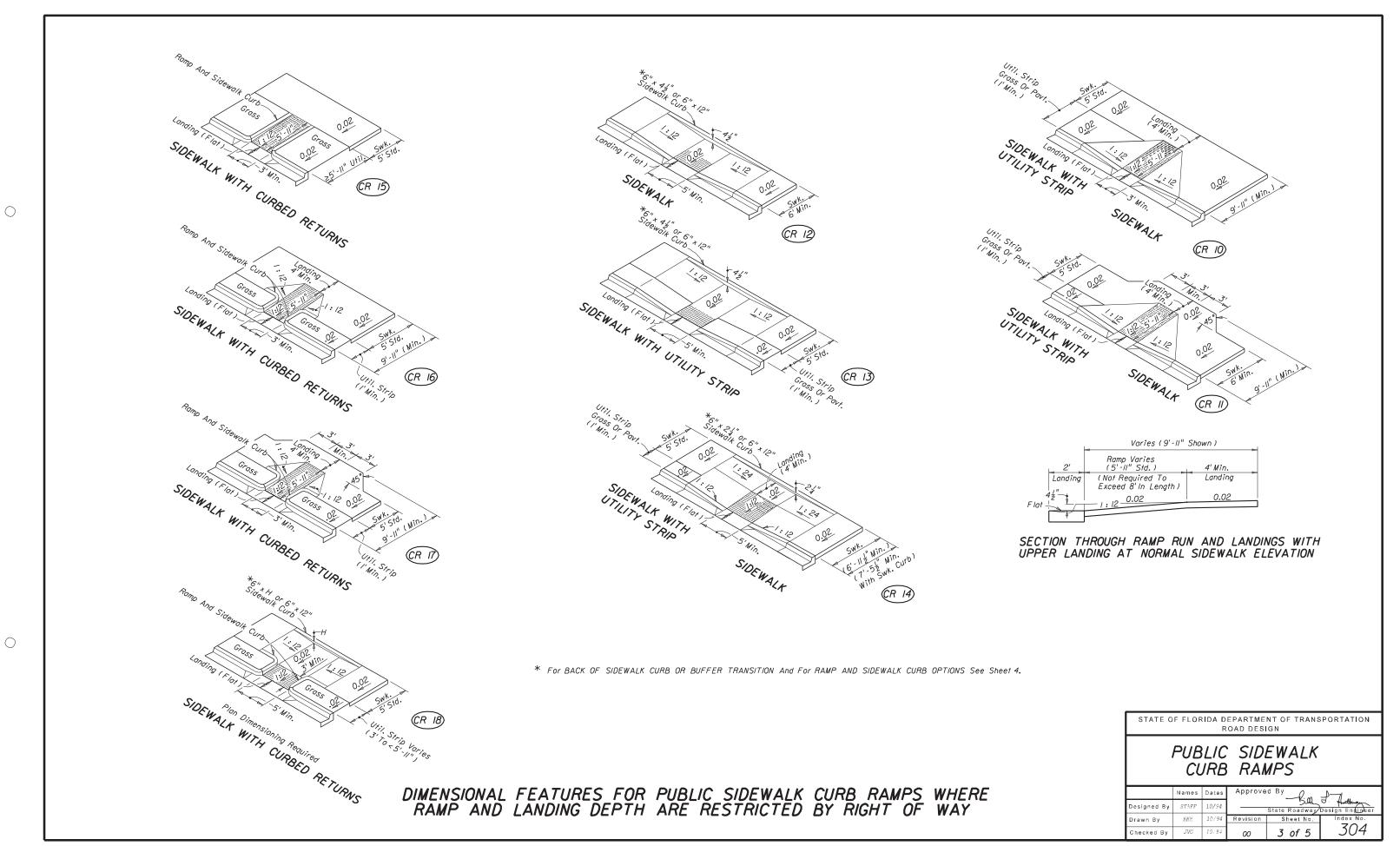
When a separate pay item for the removal and disposal of existing curb, curb and gutter, and/or sidewalk is not provided in the plans, the cost of removal and disposal of these features shall be included in the contract unit price for new curb, curb and gutter and/or sidewalk respectively.

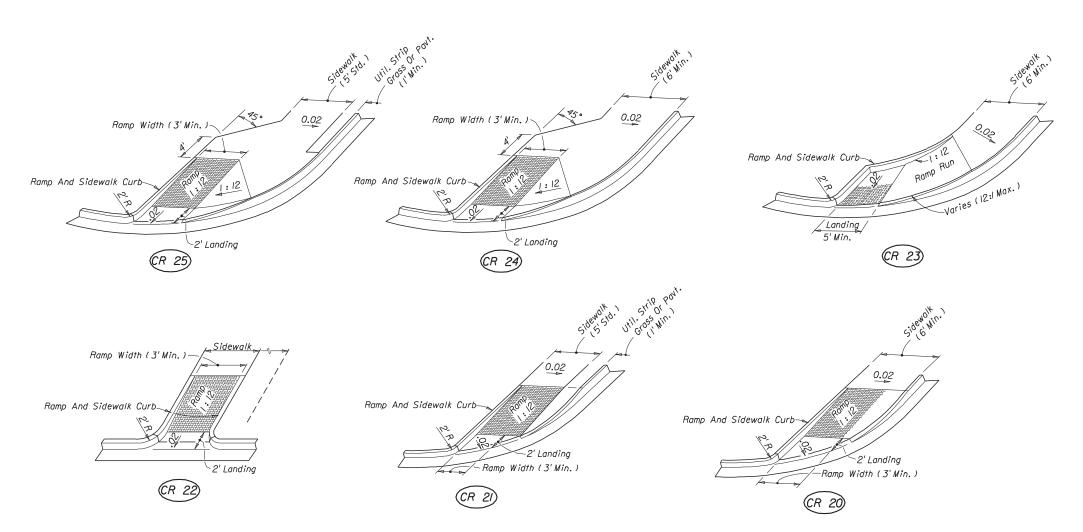
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

PUBLIC SIDEWALK CURB RAMPS

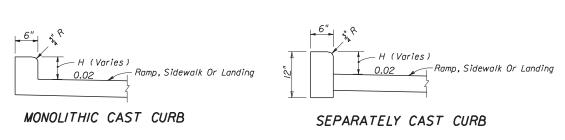
	Names	Dates	Approve		♪¬/		
Designed By	STAFF	10/94	State Roadway Design Engineer				
Drawn By	HKH	10/94	Revision	Sheet No.	Index No.		
Checked By	JVG	10/94	00	1 of 5	304		



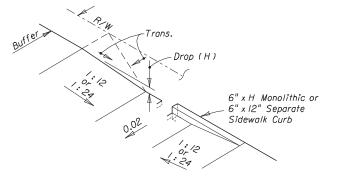




DIMENSIONAL FEATURES FOR PUBLIC SIDEWALK CURB RAMPS FOR LINEAR PEDESTRIAN TRAFFIC

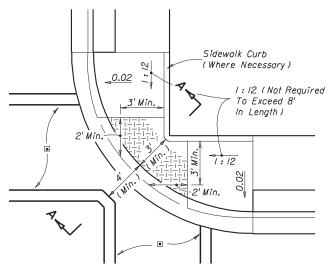


RAMP AND SIDEWALK CURB OPTIONS



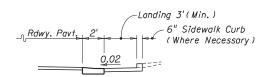
Construct Sidewalk Curb In Absence Of Adequate Buffer, Maintainable Surface Contour, Abuting Structure, Or When Called For In The Plans Or Standards

BACK OF SIDEWALK CURB OR BUFFER TRANSITION



Crosswalk width and configuration vary; must conform to Traffic Design Standards.

PLAN



SECTION AA

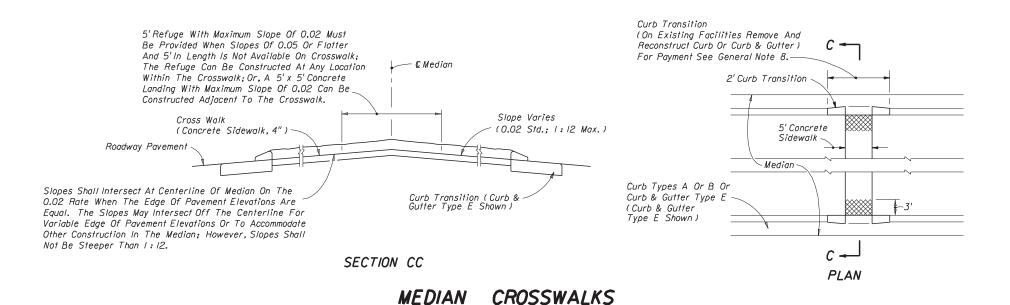


DIMENSIONAL FEATURES FOR PUBLIC SIDEWALK CORNER RAMPS UNDER CONDITIONS OF INFEASIBILITY

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

PUBLIC SIDEWALK CURB RAMPS

	Names	Dates	Approve		♪¬/		
Designed By	STAFF	10/94	State Roadway Design Engineer				
Drawn By	HKE	10/94	Revision	Sheet No.	Index No.		
Checked By	JVG	10/94	00	4 of 5	304		



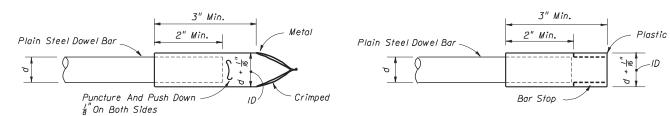
Concrete Landing O.O. Man. Stope Other Options As Shown In The Plans Concrete Landing O.O. Man. Stope Other Options As Shown In The Plans

LANDINGS FOR RAMPS WITHIN PUBLIC RIGHT OF WAY CONSTRUCTED AT LOCATIONS WHERE FUTURE SIDEWALKS ARE PROPOSED, WHERE STABLE SURFACES OTHER THAN SIDEWALKS ARE PART OF A CONTINUOUS PASSAGE OR WHERE A CURB FALLS ALONG THE CIRCULATION PATH TO PEDESTRIAN ROUTES ON ADJACENT SITES

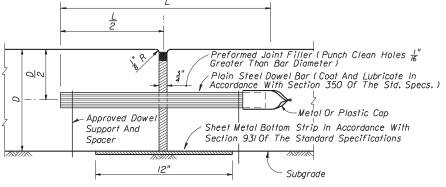
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

PUBLIC SIDEWALK CURB RAMPS

	Names	Dates	Approve	d By	♪ - /		
Designed By	STAFF	10/94	State Roadway Design Engineer				
Drawn By	HKH	10/94	Revision	Sheet No.	Index No.		
Checked By	JVG	10/94	00	5 of 5	304		

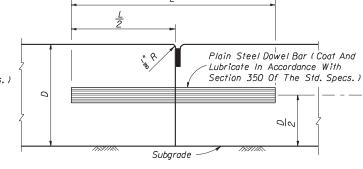


METAL OR PLASTIC CAPS FOR DOWEL BARS

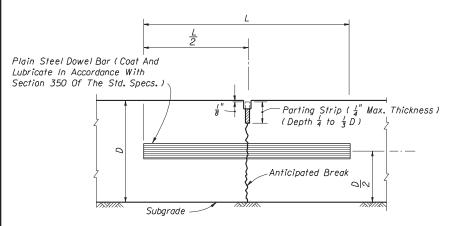


Note: Expansion joints to be placed on approaches to bridges, at street intersections and other locations indicated in detail plans.

TRANSVERSE EXPANSION JOINT

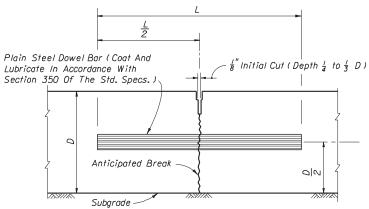


BUTT CONSTRUCTION JOINT TO BE USED AT DISCONTINUANCES OF WORK

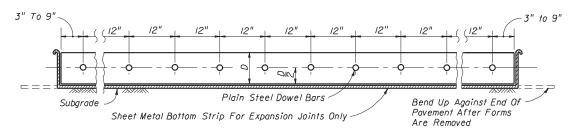


TRANSVERSE CONTRACTION JOINT, VIBRO CAST METHOD

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TRANSVERSE CONTRACTION JOINT, SAWED METHOD



DOWEL BAR LAYOUT

DOWELS (LE	NGTH 18")
Pavement Thickness "D"	Diameter
6"	3 "
7"	<i>I</i> "
8"	1"
9"	/ ¼ "
10"	14"
≥//"	1 ½ "

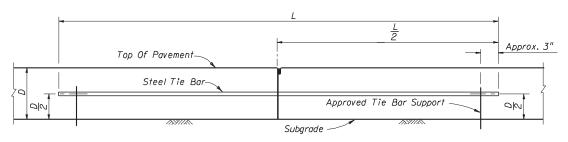
TRANSVERSE JOINTS ARE TO BE SPACED AT A MAXIMUM OF 15'. DOWELS ARE REQUIRED AT ALL TRANSVERSE JOINTS UNLESS OTHERWISE NOTED IN PLANS.

TRANSVERSE JOINTS

	MAXIMUM TIE BAR SPACING								
	Dis	tance To Clos	sest Free Edg	де					
Pavement	/.	2'	2	4'					
Thickness "D"	#4 Bars Length 25"	#5 Bars Length 30"	#4 Bars Length 25"	# 5 Bars Length 30"					
6"	48"	48"	26"	41"					
7"	45"	48"	22"	35"					
8"	39"	48"	19"	3/"					
9"	35"	48"	17"	27"					
10"	3/"	48"	/5"	24"					
//"	29"	45"	14"	22"					
/2"	26"	41"	13"	20"					
/3"	24"	38"	12"	19"					
14"	22"	35"	//"	17"					
15"	21"	33"	10"	16"					

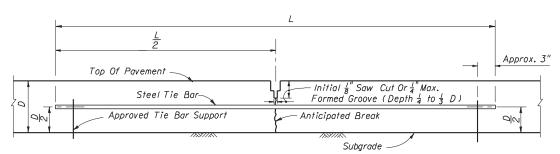
Tie bars are deformed #4 or #5 reinforcing steel bars meeting the requirements of Section 93I of the Standard Specifications.

When the distance to the closest free edge exceeds 24', provide a standard load transfer tied joint with #4 bars at 24" spacing. This joint can then be considered a free edge for determination of tie bar spacing on other joints.



Note: Tie bar spacing shall not exceed 24" at these joints.

LONGITUDINAL BUTT CONSTRUCTION JOINT



Note: Slabs poured simultaneously. Tie bars may be inserted in the plastic concrete by means approved by the Engineer.

LONGITUDINAL LANE-TIE JOINT

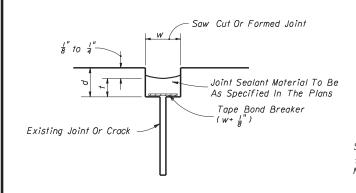
LONGITUDINAL JOINTS

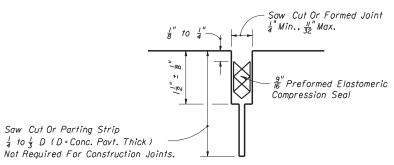
Note: For joint seal dimensions see Sheet 2.

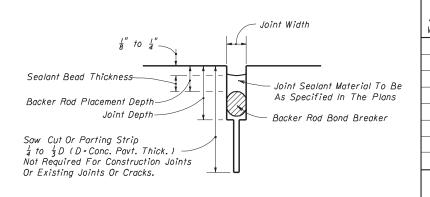
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

CONCRETE PAVEMENT JOINTS

	Names	Dates	Approve	OBY D	n.,		
Designed By			State Pavement Design Engineer				
Drawn By	HW	08/57	Revision	Sheet No.	Index No.		
Checked By	HEC	08/57	00	I of 4	305		







BACKER ROD BOND BREAKER (CONCRETE-CONCRETE JOINTS)

	JOINT	DIMENSIONS	(INCHES	J
JOINT WIDTH	SEALANT BEAD THICKNESS	BACKER ROD DIAMETER	MINIMUM JOINT DEPTH	BACKER ROD PLACEMENT DEPTH
<u> </u>	<u> </u>	3 8	1	1/2
<u>3</u> 8	<u> </u>	<u>!</u>	/ /	<u>!</u>
1/2	<u> </u>	<u>5</u> 8	/ /	1/2
<u>5</u>	<u>5</u> 16	<u>3</u> 4	/ <u>/</u> 2	9 16
3 4	<u>3</u>	1	/ 3	<u>5</u> 8
<u>7</u>	<u>7</u> 16	/ 	/ 3	<u> </u> 6
/	1/2	1 1/4	2	<u>3</u>
> /	<u>/</u> 2	1/4 +	2+	3 4

Unless otherwise indicated on the plans the joint width for new construction will be $\frac{1}{4}$ " for construction joints, $\frac{3}{8}$ " for all other joints.

For rehabilitaion projects the joint width will be shown on the plans or established by the Engineer based on field conditions.

Note: Dimension w will be shown in the plans or established by the Engineer based on field conditions. Dimension d will be constructed so that the shape factor \(\frac{\pi}{t} \) has a maximum value of 2.0 and a mimimum value of 1.0.

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FOR REHABILITATION PROJECTS

TAPE BOND BREAKER

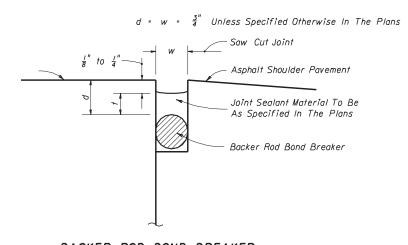
FOR NEW PROJECTS

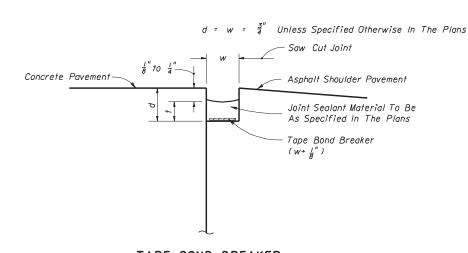
PREFORMED ELASTOMERIC COMPRESSION SEAL

FOR NEW AND REHABILITATION PROJECTS

BACKER ROD BOND BREAKER

CONCRETE - CONCRETE JOINTS





BACKER ROD BOND BREAKER

TAPE BOND BREAKER

FOR NEW AND REHABILITATION PROJECTS; EITHER TAPE OR BACKER ROD BOND BREAKER REQUIRED; SHOULDER MUST BE REPAIRED IF PROPER JOINT SHAPE CAN NOT BE ATTAINED

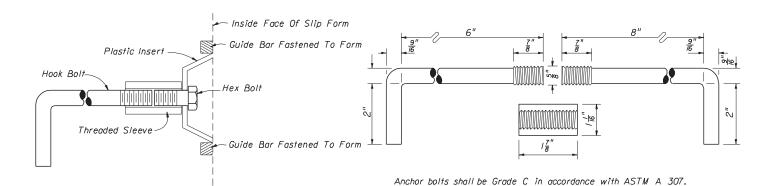
CONCRETE - ASPHALT SHOULDER JOINTS

JOINT SEAL DIMENSIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

CONCRETE PAVEMENT JOINTS

	Names	Dates	Approve		7.4.
Designed By	WIII.	05/86			Design Engineer
Drawn By	HSD	05/86	Revision	Sheet No.	Index No.
Checked By	JVG	05/86	00	2 of 4	305



NOTE: After the concrete has set to the extent that the Keyway will retain its shape, the hex bolt and plastic insert shall be removed. The remaining portion of the hook bolt assembly shall be installed immediately prior to placing of concrete in the adjacent lane.

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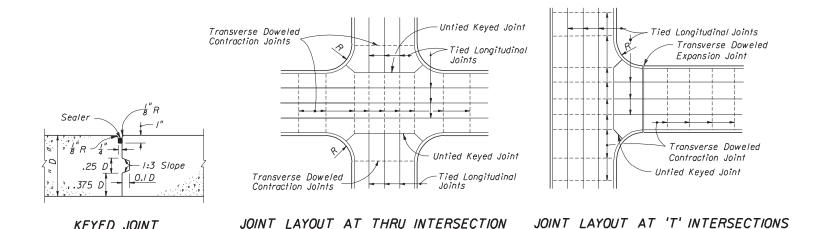
ALTERNATE KEYWAY AND HOOK BOLT

KEYED JOINT

STEEL HOOK BOLT ASSEMBLY

Threaded sleeves shall develop the full strength of the bolt and meet the

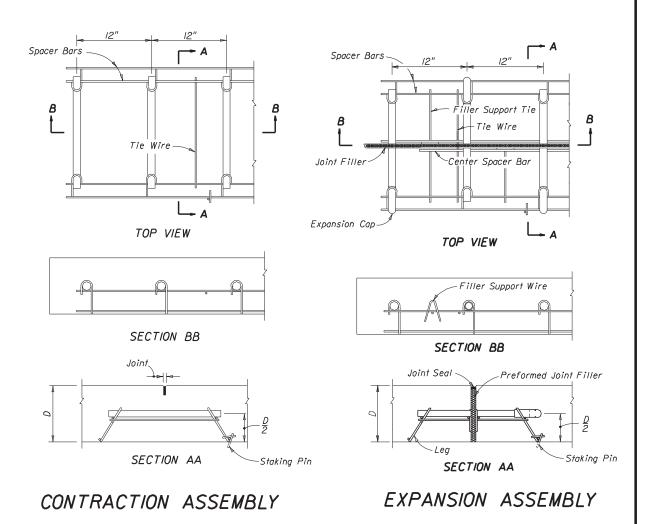
material and thread requirements of ASTM A 563.



JOINT LAYOUT AT THRU INTERSECTION

- I. Longitudinal joints will not be required for single lane pavement 14' or less in width. For entrance and exit ramp joint details, see Sheet 4 of 4.
- 2. Arrangement of longitudinal joints are to be as directed by the Engineer.
- 3. All manholes, meter boxes and other projections into the pavement shall be boxed-in with $\frac{1}{2}$ " preformed expansion joint material.

JOINT ARRANGEMENT

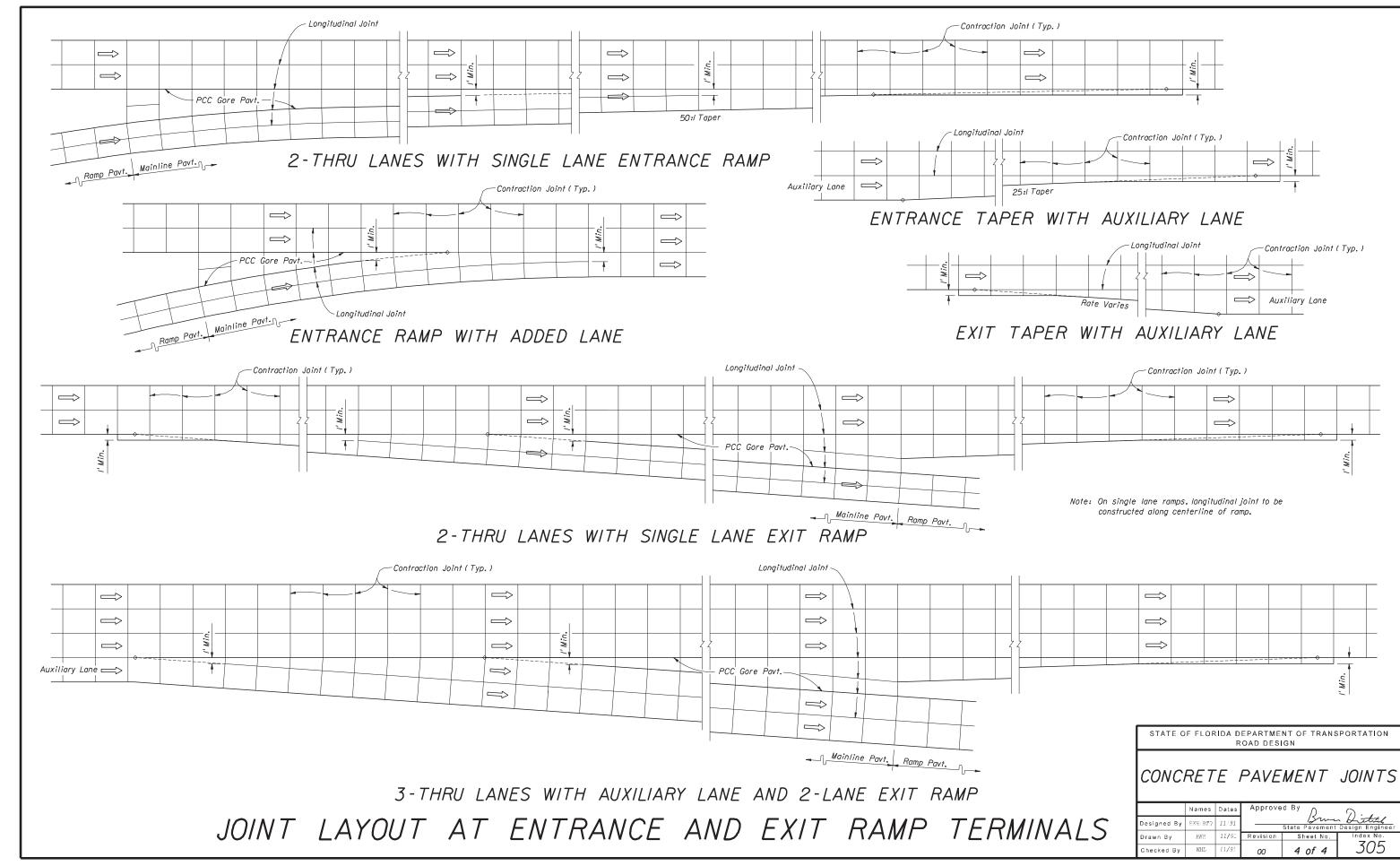


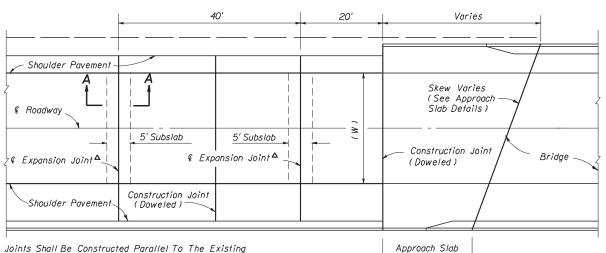
Proprietary contraction and expansion assemblies may be used. Products shall be introduced to the State Construction Office in accordance with section (C) of the Product Evaluation Procedure.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CONCRETE PAVEMENT JOINTS

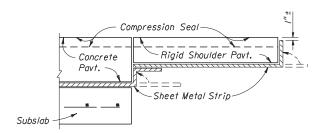
	Names	Dates	Approve	' /)	7.4.4	
Designed By	HMD	07/97	State Pavement Design Engineer			
Drawn By	HSD	07/94	Revision	Sheet No.	Index No.	
Checked By	PMD	07/97	00	3 of 4	<i>305</i>	



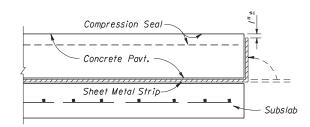


△Expansion Joints Shall Be Constructed Parallel To The Existing Transverse Pavement Joints On Rehabilitation Projects, And Parallel To The Standard Transverse Pavement Joints Shown In The Plans For New Construction.

PLAN



WITH RIGID SHOULDER PAVEMENT



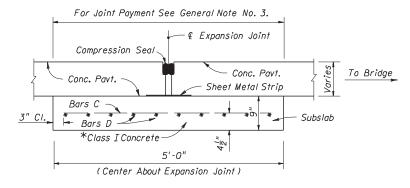
WITH GRASSED SHOULDER OR FLEXIBLE SHOULDER PAVEMENT

Note: Immediately prior to placing the seal, the joint shall be thoroughly cleaned of all foreign material. Immediately after the seal is placed, sheet metal strip shall be bent up against the pavement edge.

The sheet metal strip shall be a minimum 16 gage steel, 0.3 m wide and shall be galvanized in accordance with ASTM A-526, Coating Designation G90.

Rigid shoulder pavement shall be concrete or econocrete as called for in the plans.

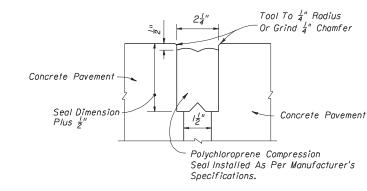
DETAIL SHOWING SHEET METAL STRIP



REINFORCING STEEL								
Mark	Size	Spac.	No. Req.		Lgth.			
С	5	6"	Varies		4'-6"			
D	5	6"	10	W	Minus	6"		

*Finish surface smooth. Cure with heavy coating of wax base white pigmented curing compound. Apply second application immediately prior to placing pavement.

SECTION AA EXPANSION JOINT



Note: All contacting surfaces between the compression seal and concrete shall be thoroughly coated with a lubricant-adhesive.

OPTIONAL SEALS

JOINT DIMENSIONS

COMPRESSION SEAL DETAIL

DESIGN NOTES

- For rehabilitation projects, the designer must indicate in the plans the number of slabs to be removed, the number of sub slabs to be constructed/reconstructed, and the location of expansion joints.
- 2. Pay quantity of expansion joint to be calculated across povement at right angles to the centerline of the roadway povement. Shoulder povement joint included.

GENERAL NOTES

- I. The centerline of roadway and the centerline of bridge do not necessarily coincide. Prior to the placement of the expansion joint, the centerline of the roadway pavement shall be determined.
- 2. For information on other types of concrete pavement joints see Index No. 305.
- Pay quantity for expansion joint is the length of joint to be constructed across the roadway and shoulder pavements, measured at right angles to the centerline of the roadway.

Payment for expansion joint shall be full compensation for joint construction, including reinforced concrete subslab, sheet metal strip and compression seal, but, not including roadway pavement reconstruction associated with joint replacement or reconstruction.

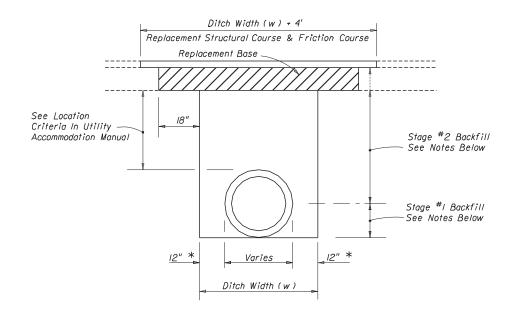
Expansion joint to be paid for under the contract unit price for Bridge Approach Expansion Joint, LF.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH EXPANSION JOINT-CONCRETE PAVEMENT

	Names	Dates	Approve		2.4.4	
Designed By			State Pavement Design Engineer			
Drawn By	LMF'	06/75	Revision	Sheet No.	Index No.	
Checked By	SFA	06/75	00	l of l	306	

\$\$\$\$\$\$\$YTIME\$\$\$\$\$



FLEXIBLE PAVEMENT NOTES

PAVEMENT REMOVAL AND REPLACEMENT

Pavement shall be mechanically sawed.

The replacement friction course shall match the existing friction course, except Structural course may be used in lieu of dense graded friction course. The thickness of the replacement asphalt pavement shall match the thickness of the existing asphalt pavement.

The new base materials shall be either of the same type and composition as the materials removed or of equal or greater structural adequacy (See Index No. 5/4).

BACKFILL

COMPACTED AND STABILIZED FILL OPTION

Backfill material shall be placed in accordance with Section 125 of the Standard Specifications.

In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.

In Stage #2, construct compacted fill along the sides of the pipe and up to the bottom of the base, with the upper I2" receiving Type B Stabilization. In lieu of Type B Stabilization, the Contractor may construct using Optional Base Group 3.

* FLOWABLE FILL OPTION

 \bigcirc

If mechanical compaction is difficult to achieve, then flowable fill may be used. When flowable fill is used, this dimension may be reduced to 4".

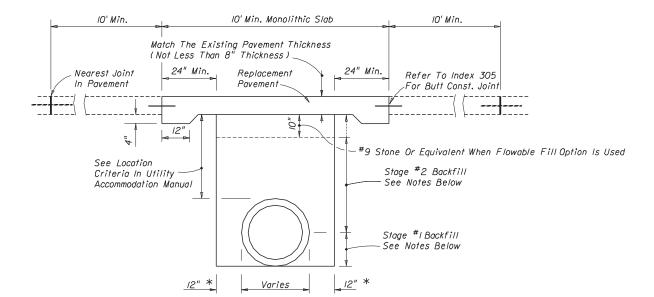
Flowable fill is to be placed in accordance with Section I2I of the Specifications, as approved by the Engineer.

If forms are used to temporarily contain flowable fill, the forms shall be in accordance with the Standard Specifications.

In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.

In Stage #2, place flowable fill to the bottom of the existing base course. Do not allow the utility being installed to float. If a method is provided to prevent floatation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.

FLEXIBLE PAVEMENT CUT



RIGID PAVEMENT NOTES

PAVEMENT REMOVAL AND REPLACEMENT

High early strength cement concrete (3000 psi) meeting the requirements of Standard Specification 346 shall be used for rigid pavement replacement.

Pavement shall be mechanically sawed and restored to conform with existing pavement joints within 12 hours. (See Index No. 305)

GRANULAR BACKFILL

Any edgedrain system that is removed shall be replaced with the same type materials. Any edgedrain system that is damaged shall be repaired with methods approved by the Engineer.

Fill material shall be placed in accordance with the Standard Specifications. Fill material shall be special select soil in accordance with Index No. 505.

In Stage #1, construct compacted fill beneath the haunches of the pipe, using mechanical tamps suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding.

In Stage #2, construct fill along the sides of the pipe and up to the bottom of replacement pavement.

* FLOWABLE FILL OPTION

If mechanical compaction is difficult to achieve, then flowable fill may be used. When flowable fill is used, this dimension may be reduced to 4".

Flowable fill is to be placed in accordance with Section I2I of the Specifications, as approved by the Engineer.

If forms are used to temporarily contain flowable fill, the forms shall be in accordance with the Standard Specifications.

In Stage #1, place flowable fill midway up on both sides of the utility. Allow to harden before placing Stage #2.

In Stage #2, place flowable fill to the bottom of the stone layer. Do not allow the utility being installed to float. If a method is provided to prevent floatation from occurring, Stages #1 and #2 can be combined, if approved by the Engineer.

RIGID PAVEMENT CUT

TRENCH CUT AND RESTORATION WITHIN ROADWAY LIMITS

GENERAL NOTES

- I. The details provided in this standard index apply to cases in which jack and bore or directional boring methods are not feasible.
- 2. Flowable fill shall not be placed directly over loose, or High Plastic, or Muck material (see Index 505) which will cause settlement due to fill weight. Where highly compressable material exists, the amount, shape and depth of flowable fill must be engineered to prevent payement settlement.
- 3. These details should not apply to utility cuts longitudinal to the centerline of the roadway which may require the additional use of geotextiles, special bedding and backfill, or other special requirements.
- 4. Method of construction must be approved by the Engineer.
- 5. Some pipe may require special granular backfill up to 6" above top of pipe. Geotextiles may be required to encapsulate the special granular material.
- 6. Where asphalt concrete overlays exist over full slab concrete pavement, the replacement pavement shall have an overlay constructed over the replacement slab. The overlay shall match the existing asphalt pavement thickness. The replacement friction course shall match the existing friction course, except Structural course may be used in lieu of dense graded friction course.

Existing broken and seated pavements shall be treated as flexible pavements.

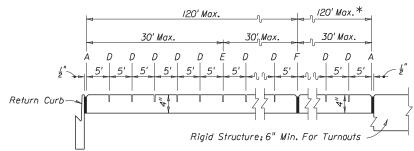
- 7. All shoulder povement, curb and curb and gutter and their substructure disturbed by utility trench cut construction shall be restored in kind.
- 8. Approved permanent patch materials may be used in lieu of structural courses.
- 9. Where long sections of flowable fill are installed, caution must be applied so local ground water flow will not be interrupted.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

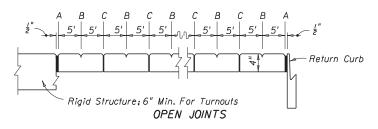
UTILITY CUT

Names | Batter | Approved By

	Names	Dates	7,661010	0 0,		
Designed By	JGW/HYD	12/95				
Designed by	000, 112	12,30	State Utilities Engineer			
Drawn By	HSD	12/95	Revision	Sheet No.	Index No.	
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Checked By	HMD, JVG	12/95	00	l of l	307	



*"E" Joint(s) Required When Length Exceeds 30'
SAWED JOINTS



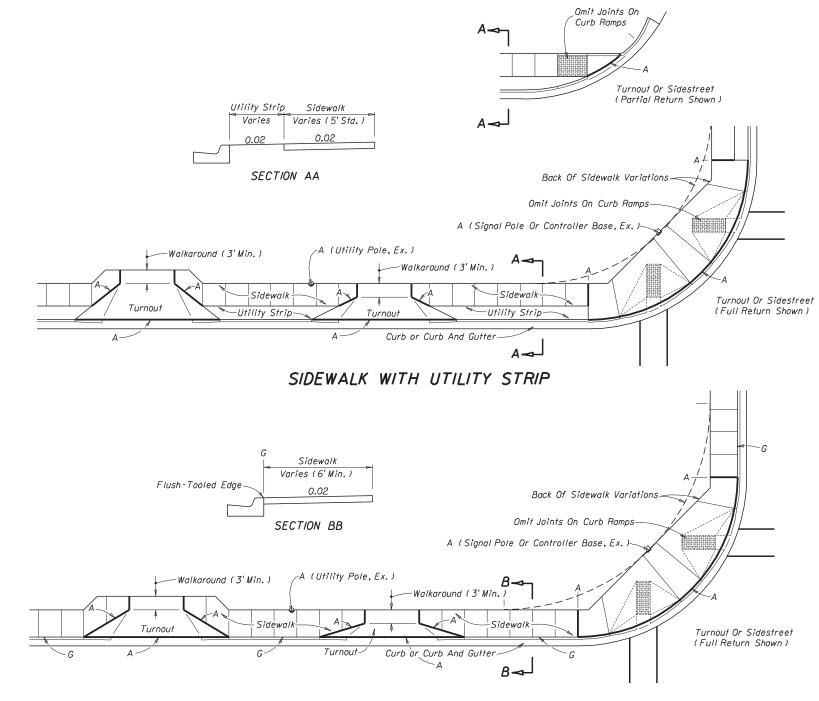
EXAGGERATED SCALE
LONGITUDINAL SECTION
SIDEWALK JOINTS

JOINT LEGEND

- A $\frac{1}{2}$ Expansion Joints (Preformed Joint Filler)
- B 1 Dummy Joints, Tooled
- C- I Formed Open Joints
- $D \frac{3}{16}$ Saw Cut Joints, $l_2^{l''}$ Deep (96 Hour) Max. 5' Centers
- $E \frac{3}{16}$ Saw Cut Joints, $\frac{1}{2}$ Deep (12 Hour) Max. 30' Centers
- F ½" Expansion Joint When Run Of Sidewalk Exceeds 120'
- G- Cold Joint With Bond Breaker, Tooled

NOTES FOR CONCRETE SIDEWALK ON CURBED ROADWAYS

- I. Sidewalks shall be constructed in accordance with Section 522 of the FDOT Standard Specifications except for public sidewalk curb ramp runs which shall be finished in accordance with Index No. 304.
- 2. Bond breaker material can be any impermeable coated or sheet membrane or preformed material having a thickness of not less than 6 mils nor more than $\frac{1}{2}$.
- 3. For public sidewalk curb ramps see Index No. 304.
- 4. For turnouts see Index No. 515.
- 5. Sidewalk shall be paid for under the contract unit price for Sidewalk Concrete (___ Thick), S.Y.



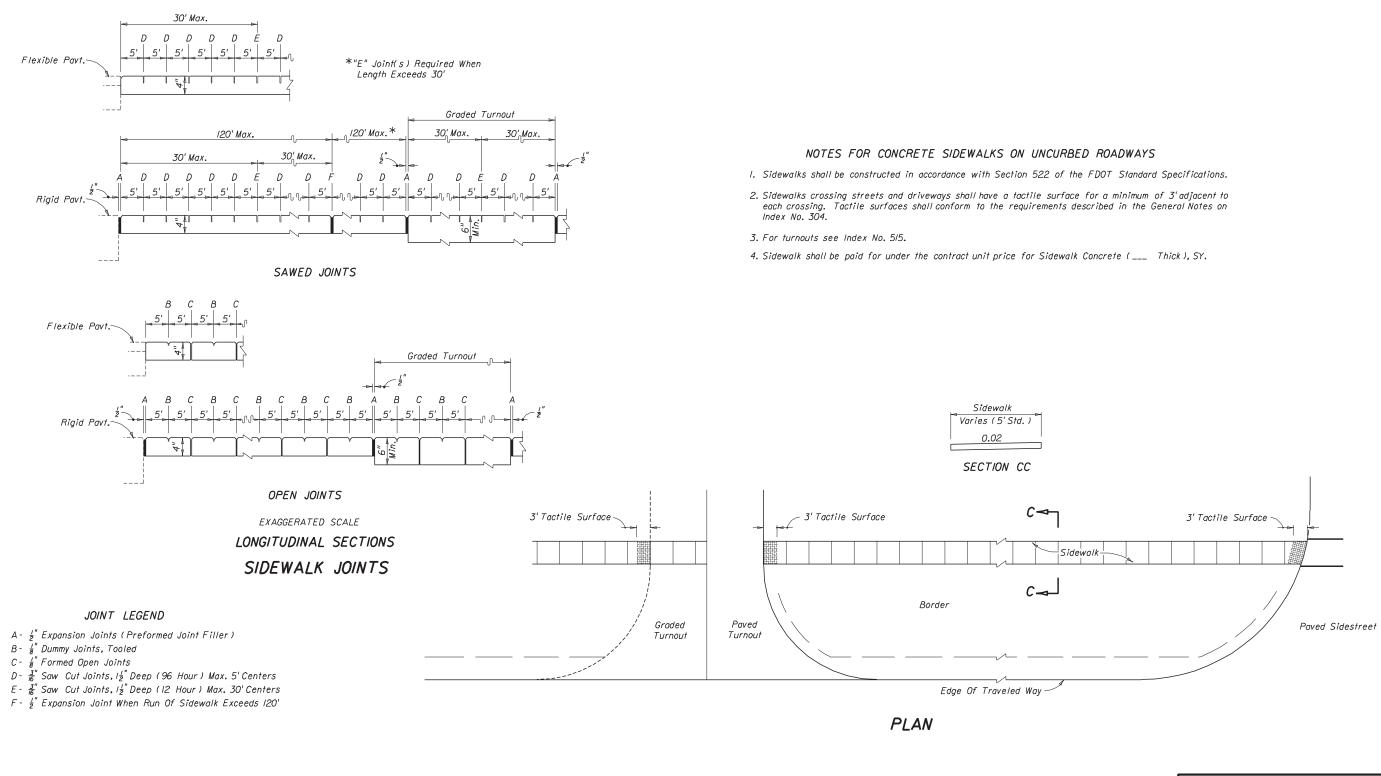
SIDEWALK WITHOUT UTILITY STRIP

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

CONCRETE SIDEWALK

		Names	Dates	Approve	d By	♪ ─/		
	Designed By	SPRCS		State Roadway Design Engineer				
ı	Drawn By	HKH	11/93	Revision	Sheet No.	Index No.		
	Checked By	JVG	11/93	00	1 of 2	3/0		

CONCRETE SIDEWALK FOR CURBED ROADWAYS



CONCRETE SIDEWALKS FOR UNCURBED ROADWAYS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

CONCRETE SIDEWALK

	Names	Dates	Approve	d By	♪¬/		
Designed By	SPECS		State Roadway/Design Engineer				
Drawn By	HKH	4/99	Revision	Sheet No.	Index No.		
Checked By	JVG	4/99	00	2 of 2	310		

GENERAL NOTES

- I. The illustrations for guardrail applications are standard configurations; adjustments are to be made as required by site specific condition to attain optimum design for function, economy and serviceability.
- 2. The beginning of guardrail need shall be at the greatest of the upstream distances from the hazard, as determined from Figure I, and other application details of this index.
- 3. One Panel equals 12'-6". Guardrail shall be constructed with rail elements 12'-6" in length except where 25'-0" elements are called for by this and other standards (indexes) or specifically called for in the plans.

Post spacings shall be 6'-3" except that reduced spacings shall be used for (a) transitions to anchorages at rigid structures such as bridges (See Details E and J) and transitions to redirective crash cushions, (b) the conditions in note no. 7 below, (c) special post applications, (d) the specific posts spacing shown in Index No. 401, (e) reduced post spacing required for specific end anchorage assemblies, and, (f) specific spacings called for in the plans.

- 4. Guardrail mounting height for the W-beam without rubrail and for thrie-beam is I'-9" to the center of beam, and for W-beam with rubrail 2'-0" to center of beam. Modified thrie-beam shall be mounted at a height of 2'-0" to center of beam. The height is critical and shall be attained in all cases; a tolerance of 3" above and I" below the standard mounting heights is permissible over necessary surface irregularities (e.g., across shoulder gutters, inlets and roadway surface break lines).
- 5. All guardrail panels, end sections and special end shoes shall be lapped in the direction of adjacent traffic.
- 6. Flared end anchorage assemblies providing 4' offset are the standard end treatments for single face free standing guardrail approach ends. Parallel end anchorage assemblies for guardrail approach end treatments will be constructed only when restraints prevent construction of flared end anchorages.
- 7. At above ground rigid hazards where the face of guardrail is offset from the hazard less than the 4' minimum for standard W-beam, other guardrail configurations may be applicable; see General Note No. 10 and the minimum offset table on Sheet 18. For guardrail with post spacing less than 6'-3" the reduced spacing should extend a minimum of one panel in advance of the hazard. When minimum offset cannot be attained safety shape concrete barrier shall be used unless other shielding is approved by the Engineer of record. See Index No. 410 for safety shape concrete barriers and typical applications, and the plans for special barrier shapes and applications.
- 8. In addition to use at conventional roadside hazards, guardrail will be required on flush shoulder sections where fill slopes are steeper than 1:3 within the clear zone, and on curbed sections where fill slopes are steeper than 1:3 within 4' of the face of curb. However, when fill heights are less than 6' the guardrail may be omitted, unless in the opinion the Engineer its use is deemed necessary due to other roadside features.
- 9. The guardrail to bridge connections contained in this index are for bridges with Test Level 4 safety shaped traffic rails. For guardrail to bridge connections on existing bridges with less than Test Level 4 rails see Index No. 401. For guardrail to concrete barrier wall connections see Index No. 410.
- 10. Thrie-beam guardrail panels shall be used in guardrail transitions to bridge traffic rail barriers, to concrete and certain water filled safety shaped barriers, certain crash cushion and as a continuous barrier when called for in the plans. For additional information on rail attachment, post spacings, nested rails, location of thrie-beam transition panels and offset block configurations see details elsewhere in this Index, and Index Nos. 410, 416 and 435. The use of thrie-beam guardrail with standard offset blocks may be considered where one or more of the conditions listed below or similar conditions are anticipated or exist:
 - a. W-beam deflection is marginal.
 - b. W-beam with rubrail considered functionally deficient.
 - c. Overriding W-beam is probable,
 - d. Drainage will be impeded or blocked by the use of concrete barrier wall,
 - e. High frequency of repairs to W-beam,
 - f. Spandrel beam with low deflection needed around unrelocatable structure, and,
 - g. Accommodating passenger vehicles heavier or larger than the standard passenger car (e.g. passenger vans and small buses)

The modified thri-beam guardrail may have application to accommodate large buses.

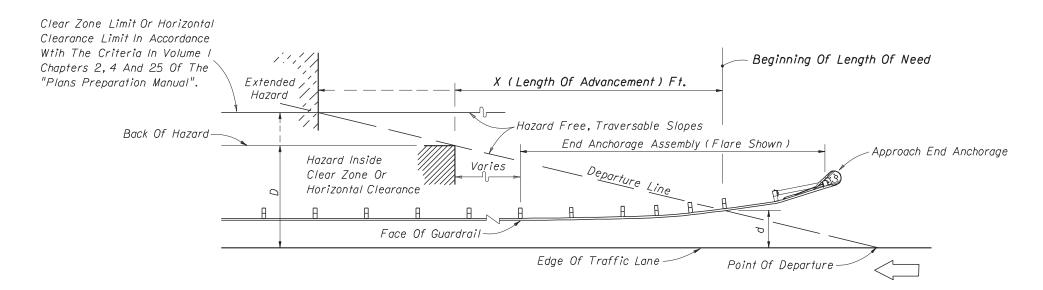
- II. Single face median guardrail for bridges located on divided roadways shall be constructed the same as outer roadway guardrail under the following conditions:
 - (a) Wide medians where approach end anchor is located outside of opposing roadway clear zone.
 - (b) Medians of uniform width that are occupied by other transportation and joint use facilities.
 - (c) Medians of uniform or variable widths with independent vertical alignments not suited to normal median guardrail installations.
 - (d) Medians of bifurcated roadways.
- 12. Straight rail sections may be used to construct radii of 125' or greater. For radii less than 125' the rail must be fabricated (shop-bent) to fit.
- 13. Crash cushions may be required in lieu of or in conjunction with guardrail at locations where space does not permit development of sufficient guardrail length, offset or crashworthiness at terminals. Crash cushions shall be constructed at or in lieu of Type II assemblies located in the approach clear zones.
- 14. Corrugated sheet steel beams, end shoes, end sections and back-up plates shall conform to the current requirements of AASHTO MI80, Class A, Type II (zinc) coating. Aluminum guardrail elements will not be permitted unless specifically called for in the plans. All other metallic components, hardware and accessories shall be in conformance with the appropriate current AASHTO requirements.

Recycled beams: Used Class A guardrail beams that have been refurbished to condition new (AASHTO MI80) may be used for both construction of new guardrail and maintenance of existing guardrail. Refurbishing shall include stripping of the existing galvanizing, restoration of the base metal in section and straightness free of warp and deformation, and, regalvanizing to AASHTO Type II specifications. Refurbished beams that retain ruptured holes, gashes or tears will not be accepted.

- 15. Steel offset blocks other than modified thrie-beam offset blocks are not permitted for new guardrail construction. Existing steel offset blocks may remain throughout the service life of the existing guardrail. Permissible post and offset block combinations are tabulated on Sheet 16.
- 16. Where necessary to enlarge or add holes to galvanized guardrail, the work will be done by drilling or reaming. Damaged galvanized guardrail will be metalized in accordance with Sections 562 and 971 of the Standard Specifications. No burning of holes will be permitted.
- 17. Guardrail reflector color (white or yellow) shall conform to the color of the near lane edgeline.
- 18. Any run of guardrail with existing concrete posts that is being reset under a consturction or maintenance contract shall be reset using timber or steel posts. Repair within a run of guardrail with existing concrete posts can be made with either steel, timber, sound salvaged concrete posts; replacement in kind of damaged posts is to be made when like posts are on hand at time of repair.
- 19. Substitutions between thrie-beam guardrail and concrete barrier wall are not eligible for V.E.C.P. consideration.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By O	D-71
Designed By				State Roadway	Design Engineer
Drawn By	HSD	8/83	Revision	Sheet No.	Index No.
Checked By	JVG	8/83	00	1 of 31	400



Design Speed mph	X (Length Of Advancement) Ft.
≤ 45	= 16 (D-d)
≥ 50	= 13 (D-d)

Length of advancement determined from the diagram and equations above establishes the location of the upstream beginning length of need for guardrail, however, the length of advancement can be no less than that required by other details of this index.

The flared end anchorage with 4' nose offset is shown in the diagram above, however, the diagram applies to other configurations that may occur at the beginning of length of need, such as, other flare designs; upstream returns; and, other upstream deflected, tangent and curvilinear conditions.

Equation Variables:

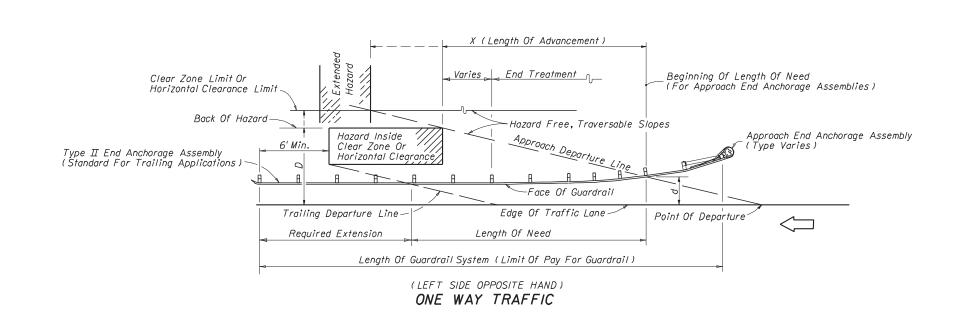
D=Distance in feet from near edge of the near approach traffic lane to either (a) the back of hazard, when the hazard is located inside the clear zone or horizontal clearance or (b) the clear zone or horizontal clearance outer limit, when the hazard extends to or goes beyond the clear zone or horizontal clearance limit. For left side hazards on two-way undivided facilities, D is measured from the inside edge of the near approach traffic lane (see Figure 2).

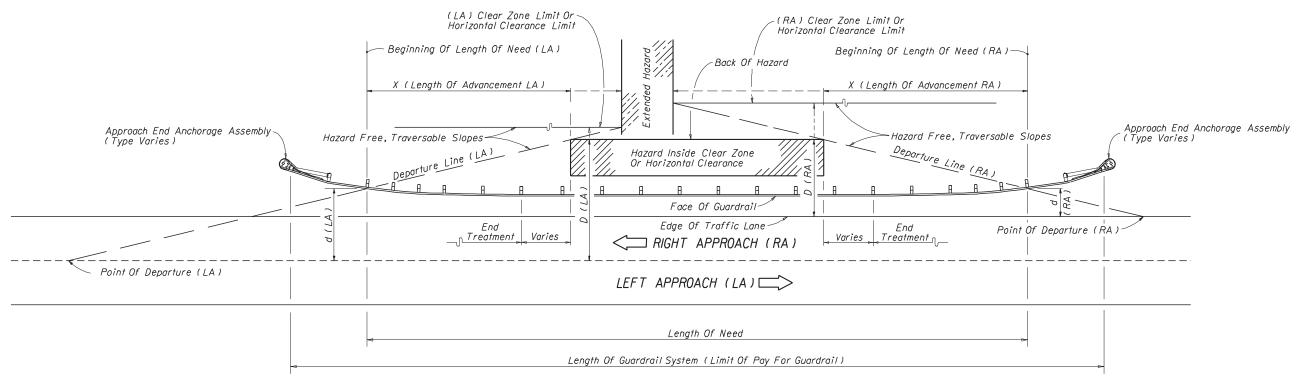
d = Distance in feet from the near edge of the near approach traffic lane to the face of guardrail at its intersection with the departure line. For left side hazards on two-way undivided facilities, d is measured from the inside edge of the near approach traffic lane (see Figure 2).

LENGTH OF ADVANCEMENT - FIGURE I

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve	d By	₽ - 71			
Designed By			State Roadway Design Engineer					
Drawn By	HSD	8/83	Revision	Sheet No.	Index No.			
Checked By	JVG	8/83	00	2 of 31	400			





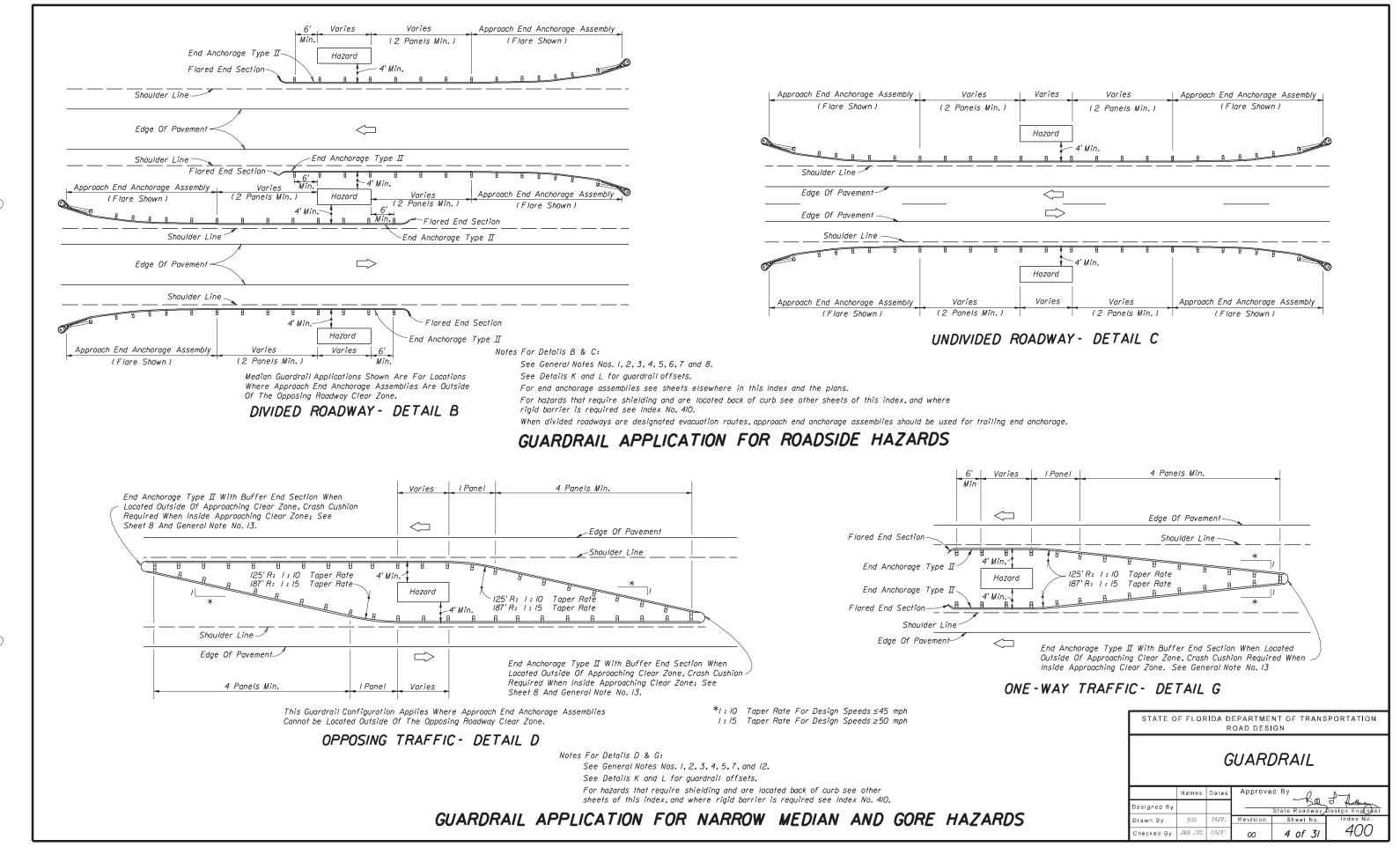
TWO LANE-TWO WAY TRAFFIC

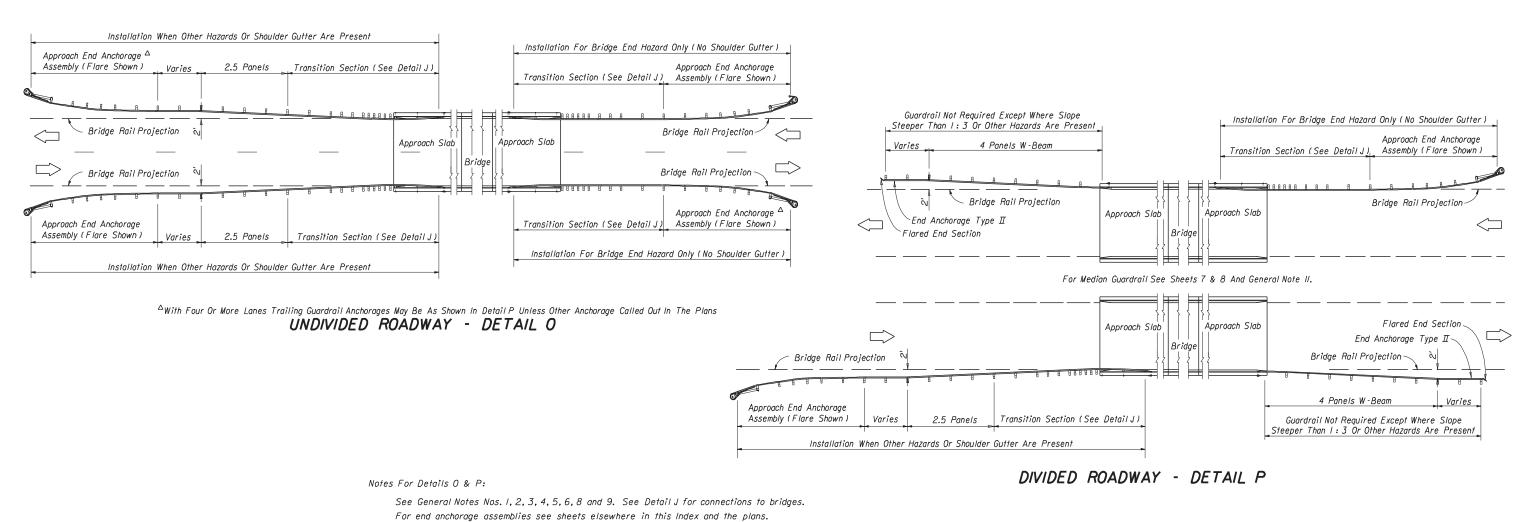
For description of the dimensions D, d and X, see Length of Advancement - Figure I. For additional shoulder guardrail information, see Details B and C.

LOCATING TERMINALS ON SHOULDER GUARDRAILS - FIGURE 2

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By	D1			
Designed By			State Roadway Design Engineer					
Drawn By	HSD	8/83	Revision	Sheet No.	Index No.			
Checked By	JVG	8/83	00	3 of 31	400			





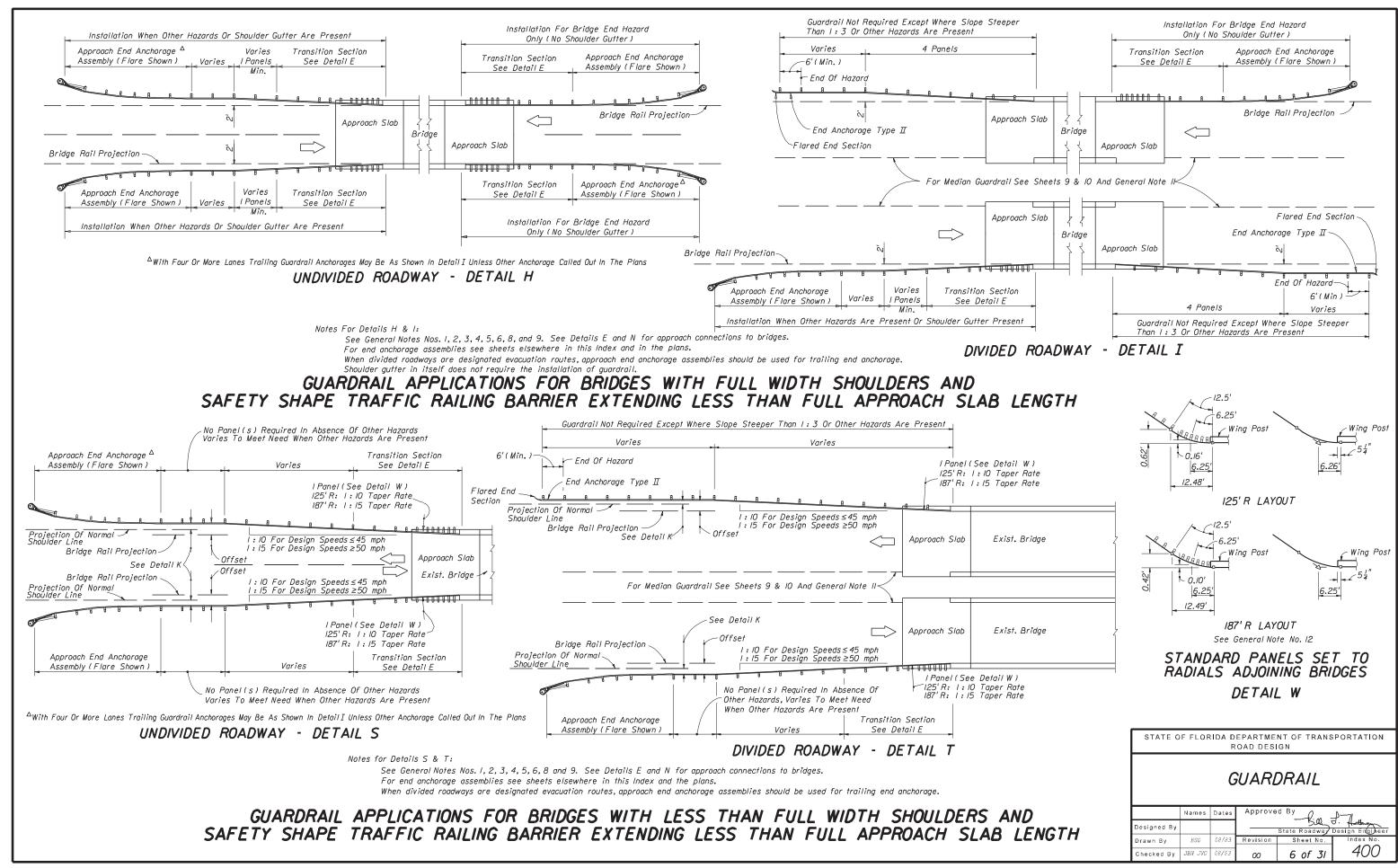
When divided roadways are designated evacuation routes, approach end anchorage assemblies should be used for trailing end anchorage.

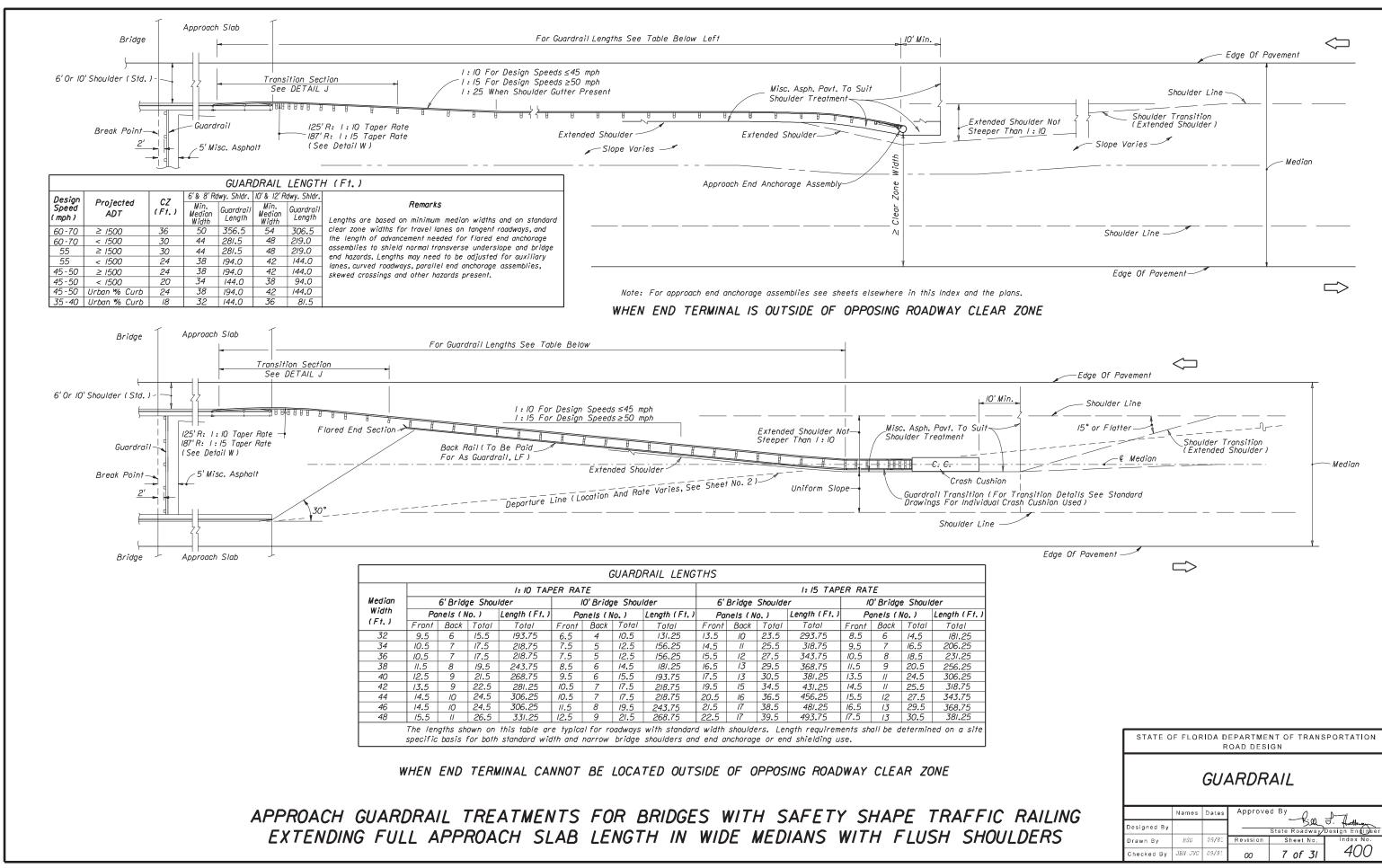
Shoulder gutter in itself does not require the installation of guardrail.

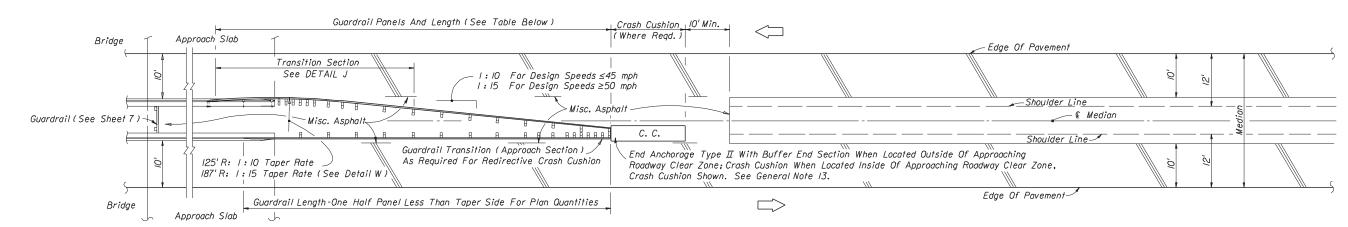
GUARDRAIL APPLICATIONS FOR BRIDGES WITH FULL WIDTH SHOULDERS AND SAFETY SHAPE TRAFFIC RAILING BARRIER EXTENDING FULL LENGTH OF APPROACH SLAB

GUARDRAIL Names Dates Designed By hecked By 00 5 of 31

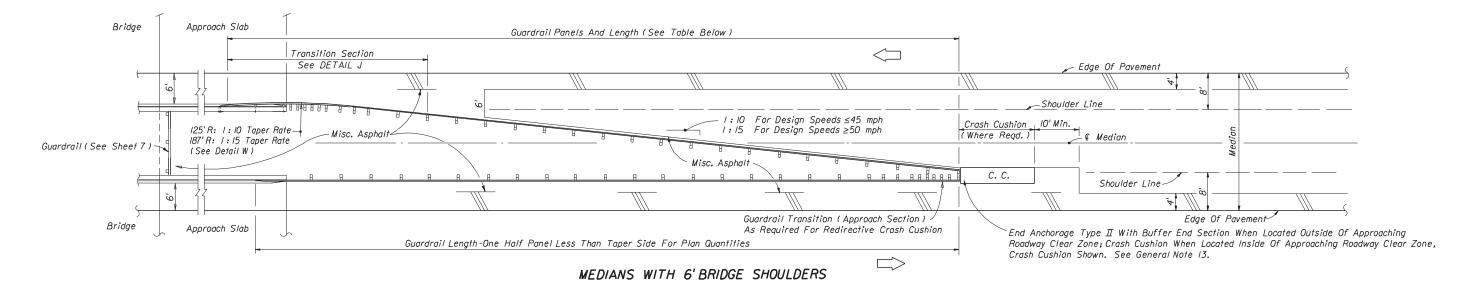
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

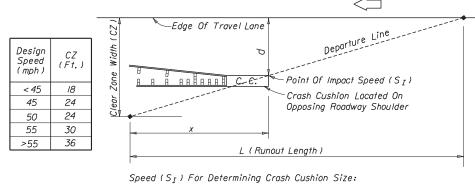






MEDIANS WITH 10' BRIDGE SHOULDERS





 $S_I = \frac{x}{L} \text{ (Design Speed)} = \frac{(CZ-d)}{CZ} \text{ [Design Speed]}$

SIZING CRASH CUSHIONS LOCATED ON OPPOSING ROADWAY SHOULDERS

Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

	GUARDRAIL LENGTHS										
	6' BRIDGE SHOULDERS				10' BRIDGE SHOULDERS						
MEDIAN WIDTH	I: 10 TAPER RATE		I: 15 TAPER RATE		I: 10 TAPER RATE		I: 15 TAPER RATE				
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)			
30	14.5	181.25	20.5	256.25	7.5	93.75	10.5	<i>131.25</i>			
28	<i>12.</i> 5	<i>156.25</i>	18.5	231.25	6.5	81.25	8.5	106.25			
26	//.5	143.75	15.5	<i>193.</i> 75	5.5*	68.75	6.5	81.25			
24	9.5	118.75	<i>13.</i> 5	168.75	5 . 5*	68.75	5 . 5*	68.75			

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds (S_I^*s) along the runouts from the approach roadways; however, when calculated speeds (S_I^*s) are less than 30 mph; crash cushions shall be no less in size than for 30 mph, see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width, see * below.

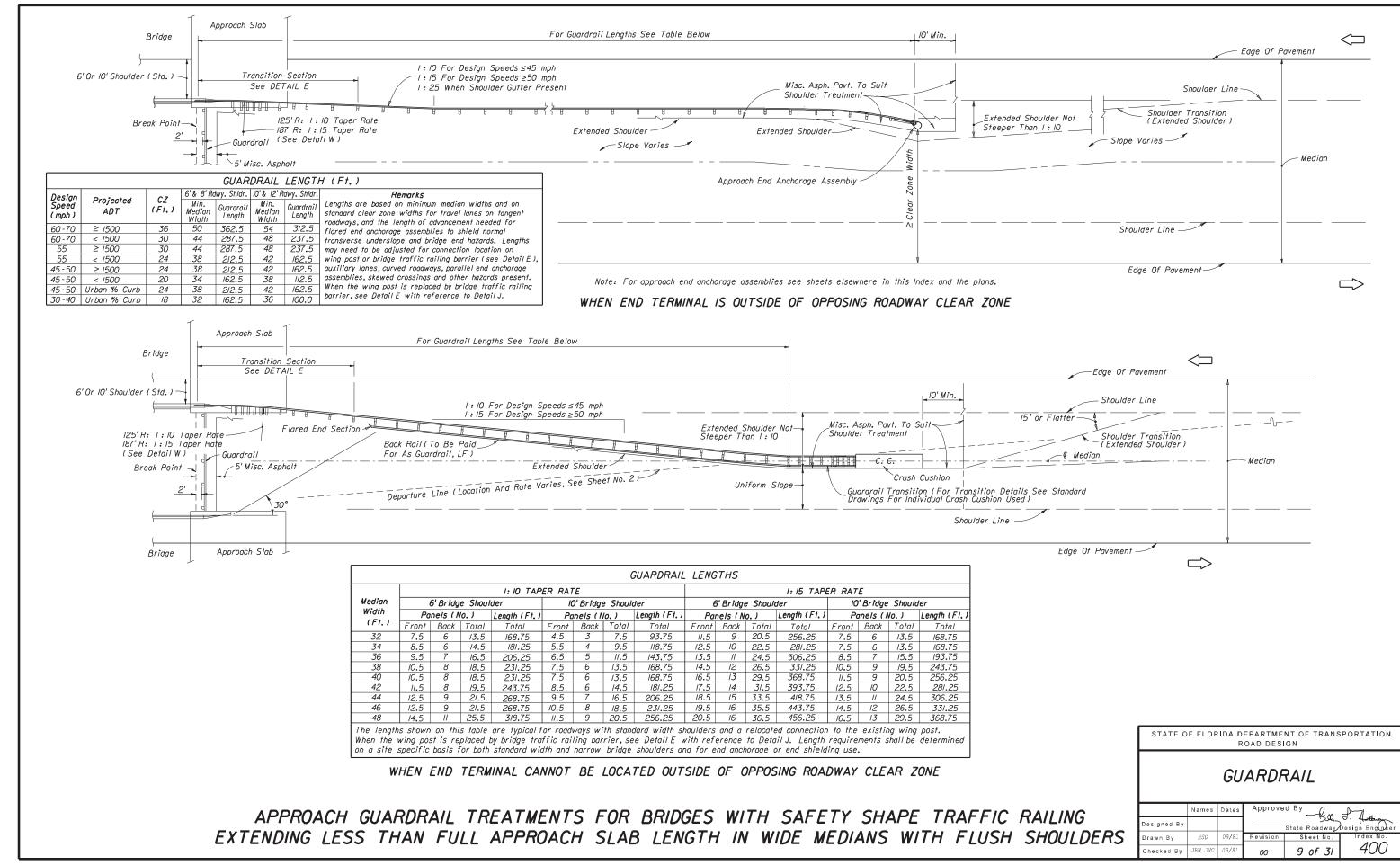
* Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

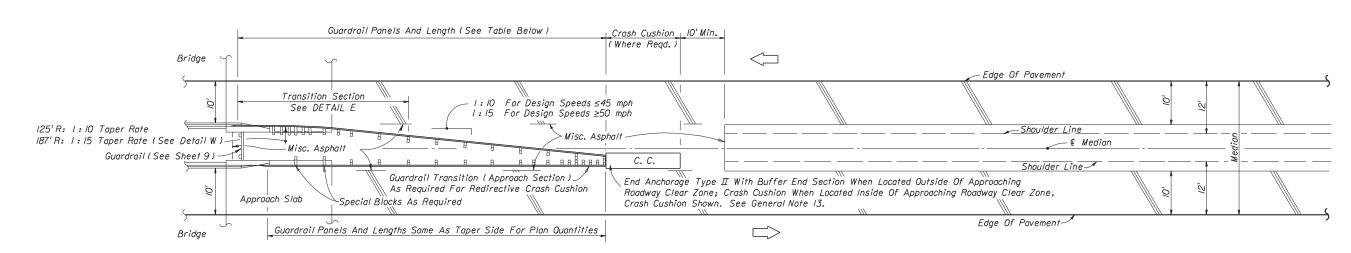
APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

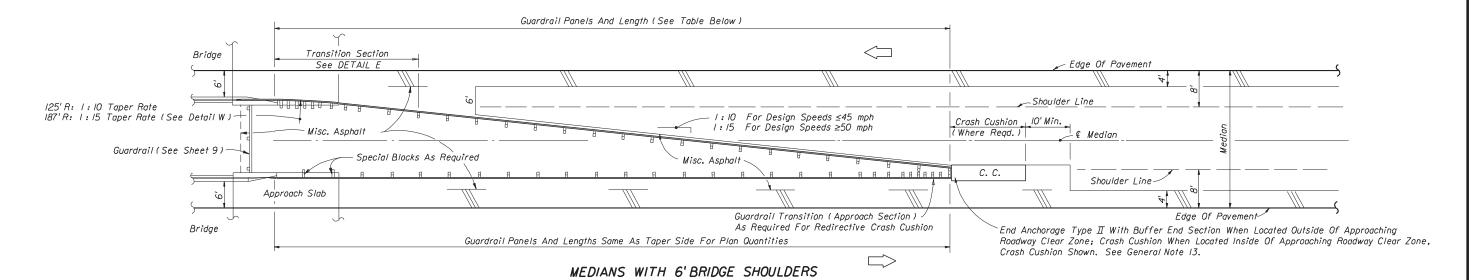
GUARDRAIL

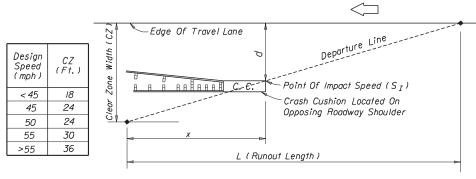
| Names | Dates | Approved By | State Roadway Design Englished By |





MEDIANS WITH 10' BRIDGE SHOULDERS





 $Speed \ (S_I) \ For \ Determining \ Crash \ Cushion \ Size:$ $S_I = \frac{x}{L} \ (Design \ Speed) = \frac{(CZ-d)}{CZ} \ [Design \ Speed]$

SIZING CRASH CUSHIONS LOCATED ON OPPOSING ROADWAY SHOULDERS

Note: The guardrail configurations shown apply only to parallel or near parallel bridges with open medians.

GUARDRAIL LENGTHS										
	6' BRIDGE SHOULDERS				IO' BRIDGE SHOULDERS					
MEDIAN WIDTH	I:10 TAPER RATE		1:15 TAPER RATE		I: 10 TAPER RATE		I:15 TAPER RATE			
(Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)	PANELS (No.)	LENGTH (Ft.)		
30	12.5	<i>156.25</i>	<i>18.</i> 5	231.25	6. 5	81.25	9.5	118.75		
28	// . 5	143.75	<i>16.</i> 5	206.25	5.5	68.75	7.5	<i>93.</i> 75		
26	9.5	118 . 75	14.5	<i>181.25</i>	5 . 5*	68.75	5 . 5*	<i>68.</i> 75		
24	8. 5	106.25	II . 5	<i>143.</i> 75	5.5*	68.75	5 . 5*	68.75		

The lengths shown in this table are based on standard widths for roadway and bridge median shoulders. Length requirements for both standard width and narrow bridge shoulders and end anchorage or end shielding requirements shall be determined on a site specific basis. When crash cushions are required on opposing roadway shoulders, their sizes may be determined by the residual speeds (S_I 's) along the runouts from the approach roadways; however, when calculated speeds (S_I 's) are less than 30 mph crash cushions shall be no less in size than for 30 mph; see speed diagram left. The number of panels may be reduced when installing a crash cushion more than 2.5' in width; see * below.

*Number shown is the minimum number of panels plus a W-Thrie beam transition panel; single faced guardrail must have a length of five (5) or more panels.

APPROACH GUARDRAIL TREATMENTS FOR BRIDGES WITH SAFETY SHAPE TRAFFIC RAILING EXTENDING LESS THAN FULL APPROACH SLAB LENGTH IN NARROW MEDIANS WITH FLUSH SHOULDERS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By	1			
Designed By			State Roadway Design Engineer					
Drawn By	RWR	08/82	Revision	Sheet No.	Index No.			
Checked By	JVG/JBW	08/82	00	10 of 31	400			

LEGEND \bigcirc Pavement return (radius R_1). (4) Flared end anchorage to be installed except when existing guardrail on intersecting drive or side road adjoins the project.

RADIAL GUARDRAIL									
			Nor	mal i	Turnou	ıts			
		Taper				Simple (Curve		
R_I	R ₂	Panels Required		7	R ₂	Panels Required	Δ		
15'	25'	3	85°	56'	25'	3	85° 56'		
20'	25'	3	85°	56'	25'	3	85° 56′		
25'	25'	3	85°	56'	25'	3	85° 56′		
30'	25'	3	85°	56'	25'	3	85° 56′		
35'	25'	3	85°	56'	25'	3	85° 56′		
40'	40'	5	89°	3/'	40'	5	89° 31′		
45'	40'	5	89°	31'	40'	5	89° 31'		
50'	40'	5	89°	31'	40'	5	89° 3/'		

Note: Only 25' and 40' radius panels are to be used for return guardrail on normal turnouts. On skewed turnouts the number of panels used and their arrangement with straight panels will be as shown in the plans or as directed by the Engineer.

No. 2 post for Radii 25' or less. No. 3 post for Radii > 25' and <50'. Between No. 4 and No. 5 posts for Radii 50' or greater.		by
Intersecting Drive Or Side Road Post for locating flare, proximate to PC or PT: No. 3 post for Radii 25' or less. Between No. 4 and No. 5 posts for Radii greater than 25'.	. Intersecting Drive	
7 Expanded shoulder for guardrail.	Or Side Road	
(8) Expanded shoulder for flared guardrail end anchorage.	Ť	
——————————————————————————————————————		
(3)	d Varies	s $\mid \cdot \mid \cdot \mid$
Radial guardrail to be installed when guardrail required on the intersecting drive or side road (radius R_2).		
© End anchorage Type II(radial return only).	(3)	
	7 - 4	\-\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
		///
3		
9 PC or PT PC or PT Q 9		
Roadway_	PC or PT	Roadway 🥄

SIMPLE CURVE TURNOUTS

TAPER TURNOUTS

 \bigcirc

Note: The guardrail application shown on this sheet are for highways with flush shoulders and no restraints for constructing flared end anchorages and minimum lengths of guardrail. For highways with flush shoulders and restraints to constructing flared anchorages, see General Note No. 6.

Where openings in guardrail are required in close proximity to bridge traffic rails or ends of concrete barrier walls, and minimum length guardrail with flared end anchorages can not be applied, either controlled release returns or energy absorbing terminals are to be applied.

GUARDRAIL APPLICATIONS FOR INTERSECTING DRIVES AND SIDE ROADS ON RURAL FACILITIES

① Edge of roadway pavement.

(5) Post for locating flare, proximate to PC or PT: No. 2 post for Radii 25' or less.

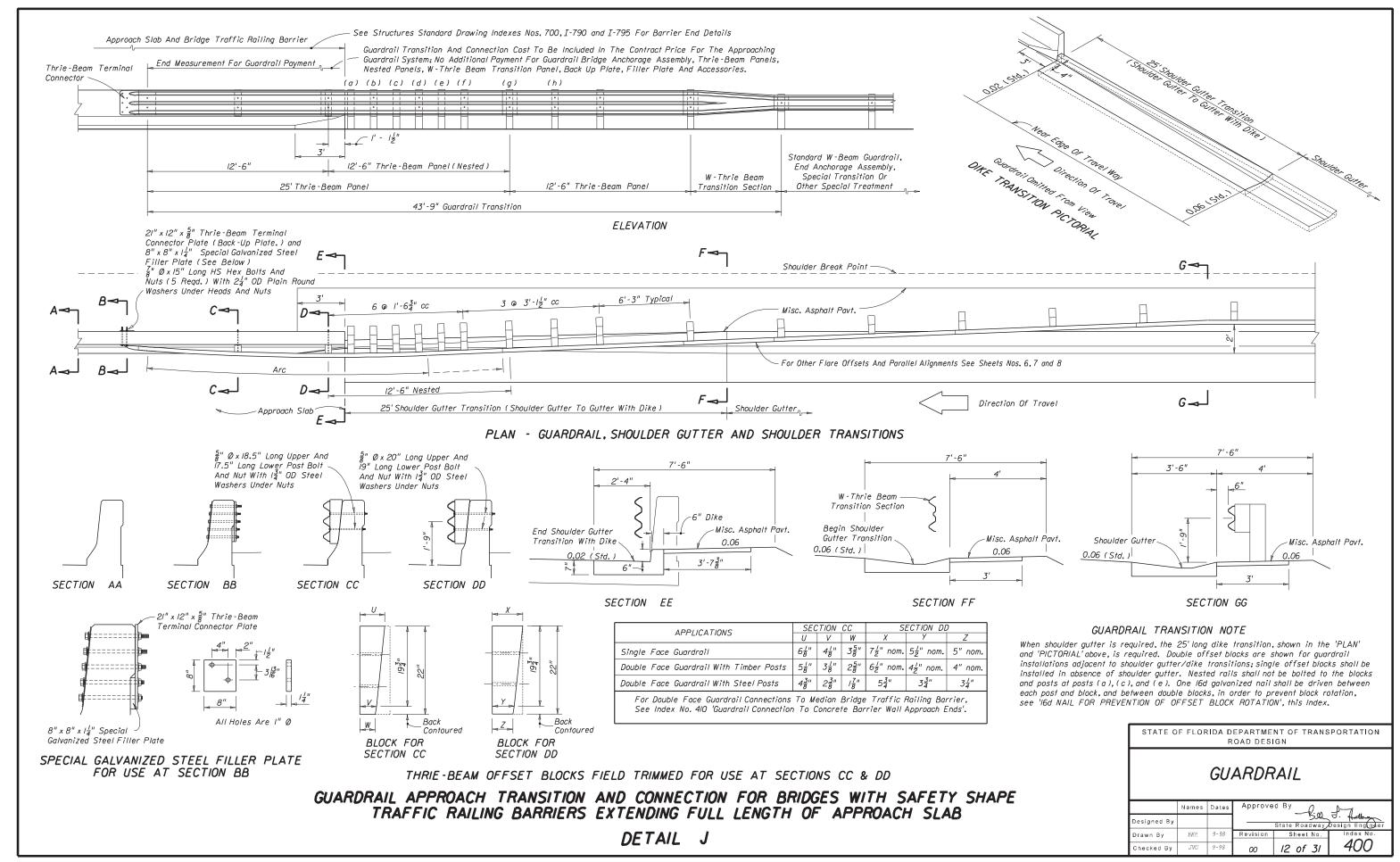
Taper.

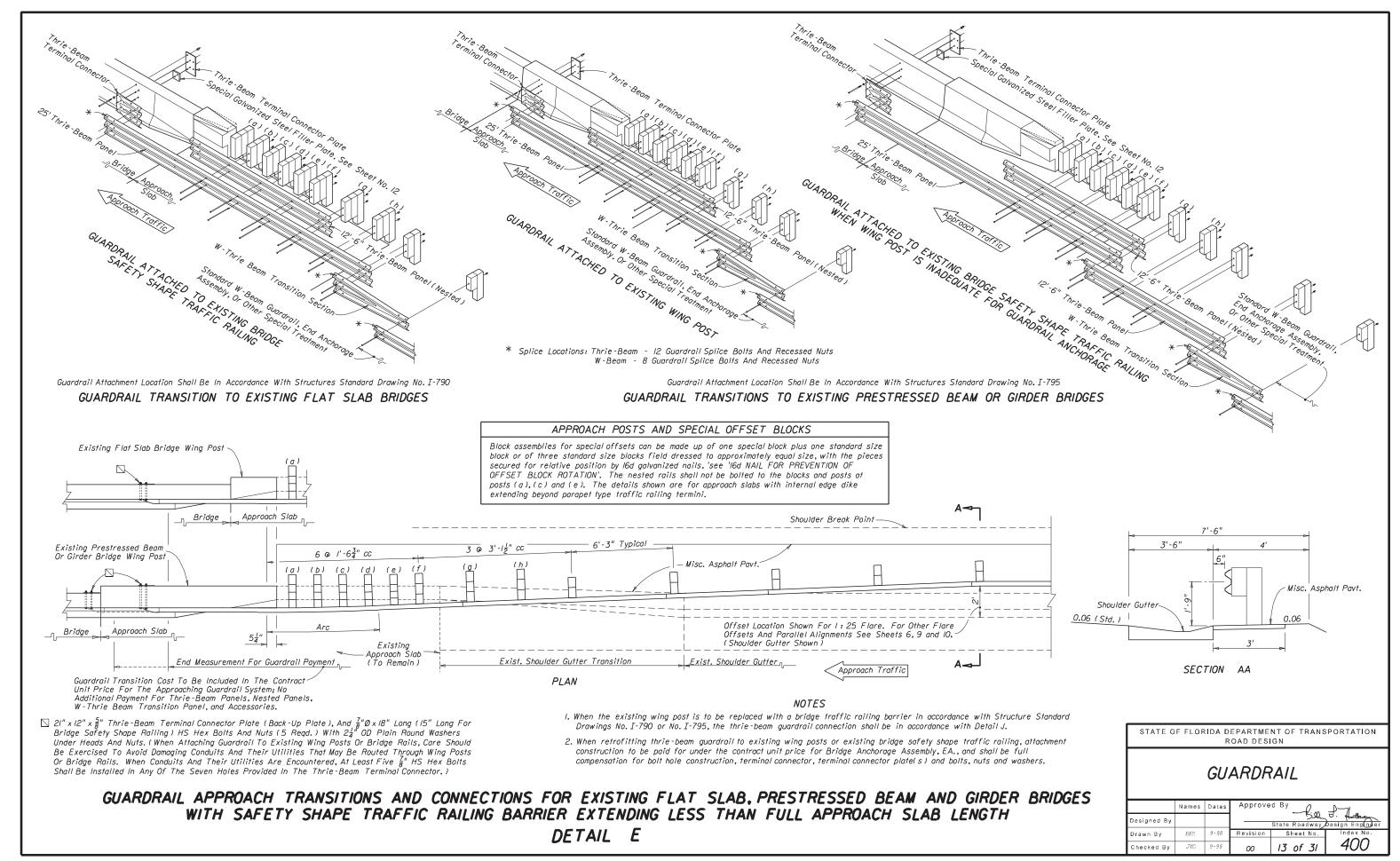
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

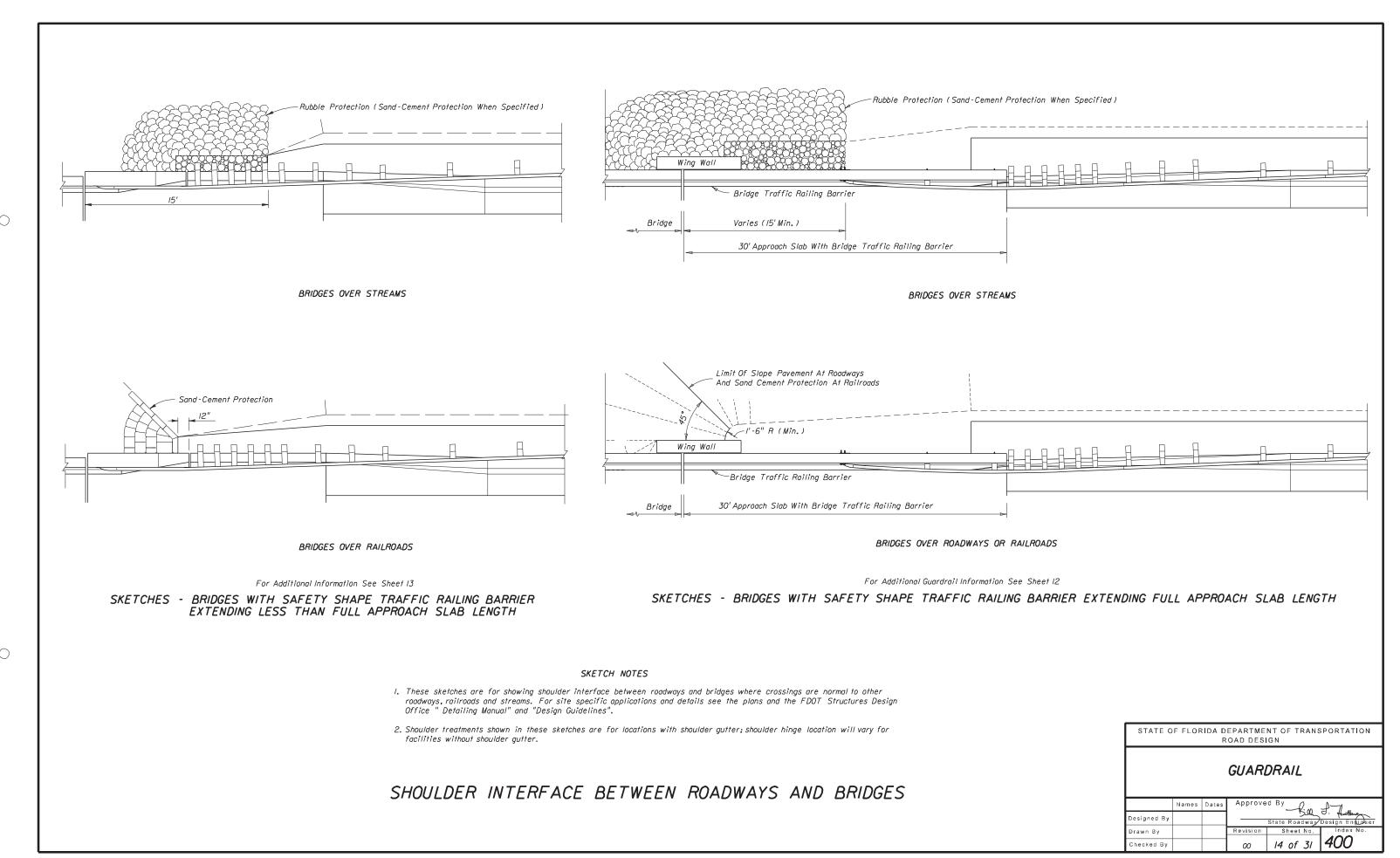
GUARDRAIL

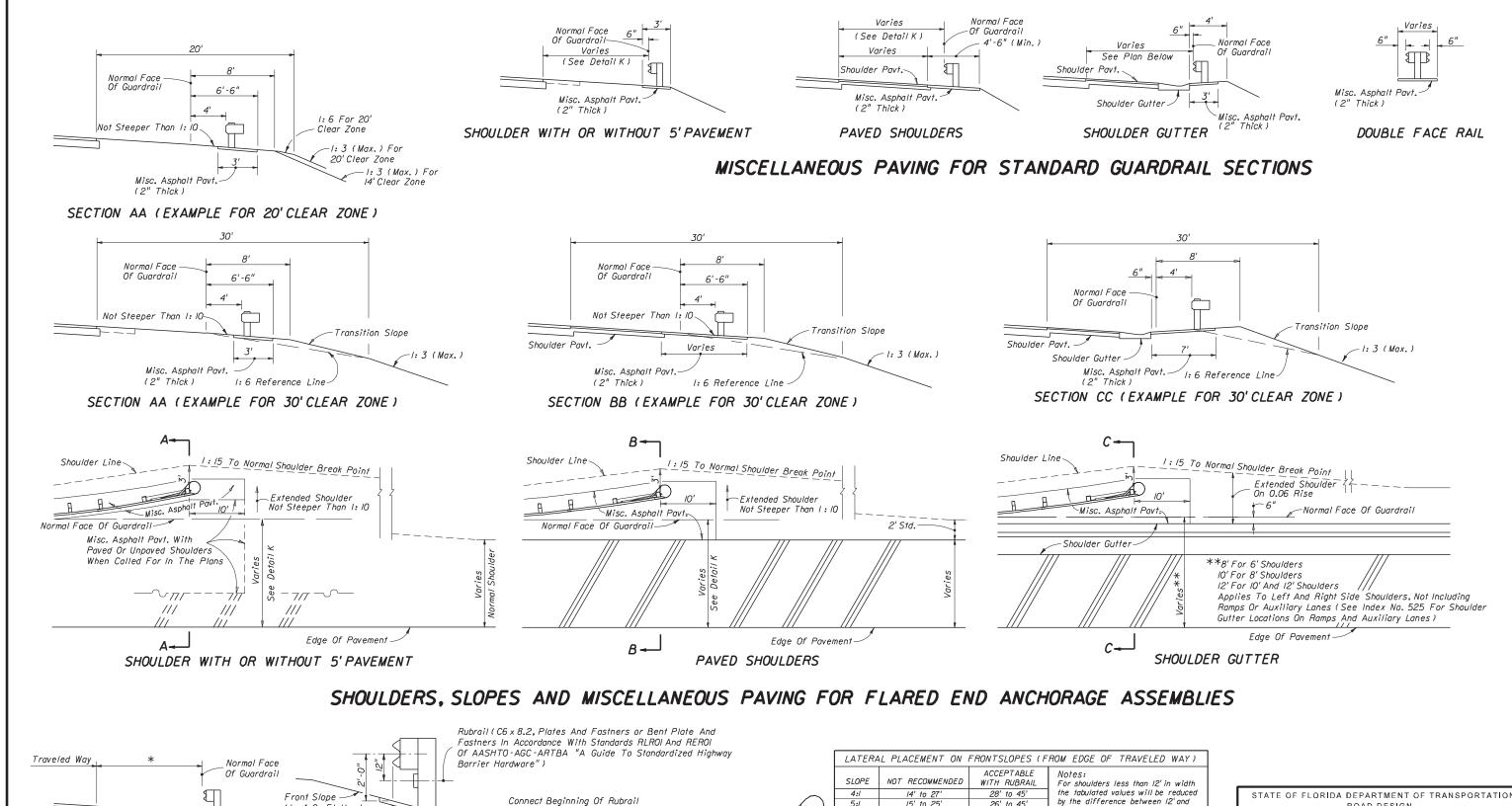
	Names	Dates	Approve	d By	A-71
Designed By			State Roadway Design Engineer		
Drawn By	HSD	09/83	Revision	Sheet No.	Index No.
Checked By	JVG	09/83	00	11 of 31	400

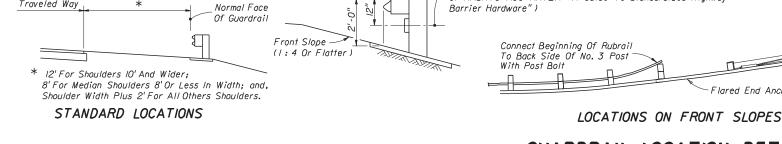
PC or PT











	SLOPE	NOT RECOMMENDED	WITH RUBRAIL	For shoulders less than 12' in width
	4:/	14' to 27'	28' to 45'	the tabulated values will be reduced
<i>(</i> -1)	5 : /	15' to 25'	26' to 45'	by the difference between 12' and
The	6:/	17' to 22'	23' to 45'	the shoulder width.
- Barrier Barrier	7:/	21' to 24'	25' to 45'	Placement of guardrail on frontslopes
	8:/	Acceptable to 25'	26' to 45'	steeper than 4:I not recommended.
lared End Anchorage	9:/	Acceptable to 26'	27' to 45'	Cost of rubrail to be included in the
_	10:1	Acceptable to 27'	28' to 45'	contract unit price for guardrail.
T CLODEC				

GUARDRAIL LOCATION - DETAIL K

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

GUARDRAIL

Approved By

Designed By

Drawn By

JM

O7/81

Revision Sheet No. Index No.
Checked By

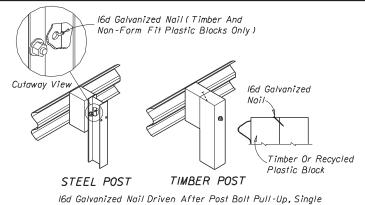
J8W/JVG

O7/81

OO

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Reflector

Pine Rail

Timber Post-

End Fixture

And Double Face Guardrail, Single Face Guardrail Shown (16d Nail Between Blocks For Multiple Offset Blocks).

Install Pipe Rail Over Pipe Rail End Fixture And Thru-bolt With $\frac{1}{2}$ " x $3\frac{1}{2}$ " Long Hex Bolts And Nuts With $\frac{1}{2}$ " Plain

-Steel Post

Round Washers Under Heads And Nuts (2 Regd.)

(Upset Threads After Tightening)

Offset Block

16d NAIL FOR PREVENTION OF OFFSET BLOCK ROTATION

 \bigcirc

REFLECTOR NOTES

- I. Reflectors shall conform to Section 993 of the Standard Specifications.
- 2. Reflector color (white or yellow) shall conform to the color of the near
- 3. Face of rail bolt, screw, rivet or bracket mounted reflectors shall not be used in lieu of adhesive mounted reflectors.
- 4. Post mounted reflectors approved on the 'Qualified Products List' may be used by FDOT Maintenance to replace damaged or missing reflector in a continuous run of existing post mounted reflectors. Adhesive and post mounted reflectors shall not be intermixed in a continuous run of guardrail.
- 5. The cost for reflectors shall be included in the contract unit price for

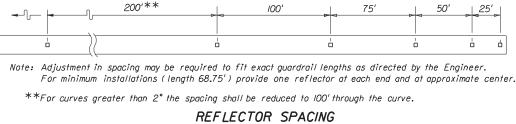
POSTS OFFSET BLOCKS REMARKS Post bolt hole in timber and plastic blocks to be centered ($\pm \frac{l''}{4}$) All timber offset blocks shall be dressed on all four sides (S4S). 6" x 8" x I4" Timber (Nominal) Or See Note I below. Timber 6" x 8" x I4" Recycled Plastic One I6d galvanized nail per block is to be used to prevent rotation of block (see detail left). Same as above for timber and plastic blocks. Form fit plastic 6" x 8" x I4" Timber (Nominal) Or Steel W6 x 9 Or 6" block holes alian with holes in steel posts and do not require 6" x 8" x I4" Recycled Plastic $\frac{1}{8}$ " Øx $\frac{1}{2}$ " long hex head bolts with full length thread and nuts MI4 x I8 x 22" Or WI4 x 22 x 22" (2 Reqd.) and $\frac{5}{8}$ " plain round washers (4 Reqd.) for mounting Steel W6 x 9 Or 6" C (Steel Modified Thrie-Beam) steel block to post. Bolts are to be installed in opposite holes. top and bottom. Notes: I. Thrie-beam offset blocks are 22" in length.

- - 2. Timber and recycled plastic offset blocks of equal size can be intermixed within a run of rail.

€ Post Bolt-

3. Rubber offset blocks are not to be used on moderate or high volume facilities and used only on low volume facilities when specifically called for in the plans

PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS



Reflector

✓ Direction Of Traffic

SECTIONAL FACE VIEW

Steel Guardrail Post

 $2\frac{1}{2}$ " x 2" x $\frac{1}{4}$ " x 4" Long

Angle Bracket (Galvanized)

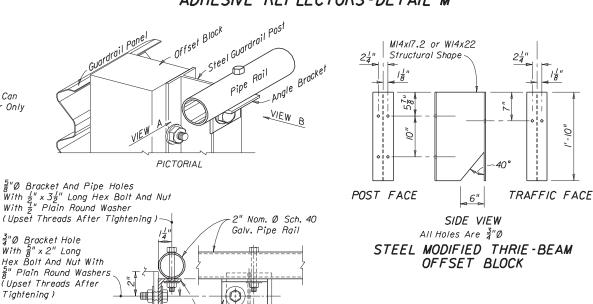
Reflectors shall be centered in the channel of

W-beam and in the top channel of thrie-beam.

REFLECTOR MOUNTING

VIEW

ADHESIVE REFLECTORS-DETAIL M



──Steel Guardrail Post

¦" Offset From ₢ Of Guardrail Post

14"

4"

VIEW B

PIPE RAIL MOUNTING

W - BEAM€ Post Bolts

€ Post Bolt

€ Post Bolt W - BEAM

W-BEAM WITH RUBRAIL

€ Post Bolts THRIE - BEAM

MODIFIED THRIE-BEAM

THRIE - BEAM *Front Slope When Right Of Way, Environmental Or Other Restrictions Prohibit Normal Shoulder Extension

DOUBLE FACED GUARDRAIL

Steel Modified Thrie-Beam

Offset Blocks (See Detail

Left And Table Above) € Panel Boli

SINGLE FACED GUARDRAIL

Rubrail When Called For

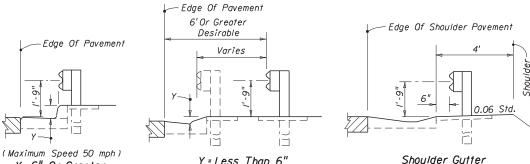
By Location In The Plans

As Specified, See Sheet 15

For Additional Information)

(On One Or Both Sides

MOUNTING HEIGHTS ON SHOULDERS AND IN MEDIANS



LOCATION AT CURB & GUTTER SECTIONS-DETAIL L

Y = Less Than 6" Y = 6" Or Greater

Names Dates Approved By Designed By State Roadway 09/8 hecked By JBW/JVG 09/8/ 00 16 of 31

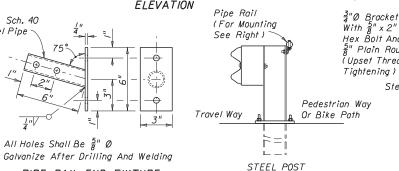
GUARDRAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SPECIAL SAFETY PIPE RAIL

\$\$\$\$\$\$\$\$\$YTIMF\$\$\$\$\$

Guardrail Beam -Attach Pipe Rail End Fixture To Post With $\frac{1}{2}$ " x 7" Long Hex Bolts And Nuts With $\frac{1}{2}$ " Plain Round Washers Under Heads And Nuts (2 Read.) This Post Can (Upset Threads After Tightening) Be Timber Only Steel Post Guardrail Beam 20"± R Pipe Rai Top Of Curb-Lip Of Gutter Pipe Rail End Fixture Gutter -ELEVATION Pipe Rail $l_2^{l''}$ 0 Sch. 40 Steel Pipe \searrow (For Mounting See Right)



PIPE RAIL END FIXTURE

PIPE RAIL NOTES

I. Special Safety Pipe Rail required on steel guardrail posts when pedestrian ways and bikeways are located 4' or less from back of the posts. Begin and end the pipe rail in accordance with this detail.

SECTION

2. When guardrails with timber posts are located with the back of posts 4' or less from the near edge of the pedestrian way or bikeway, the bolt ends will require one of the following treatments:

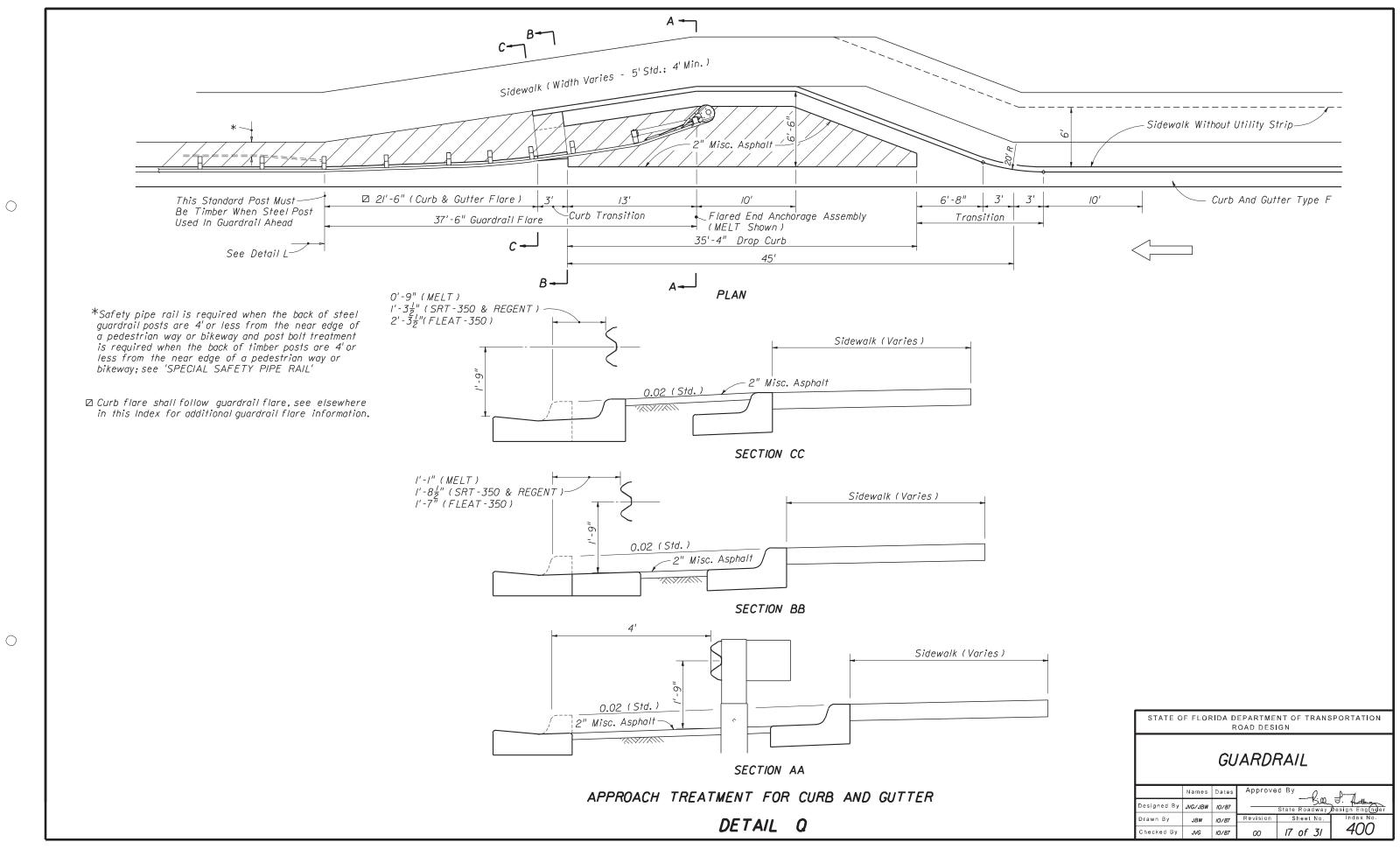
(a) Trimming back flush with the face of nut and metalizing or

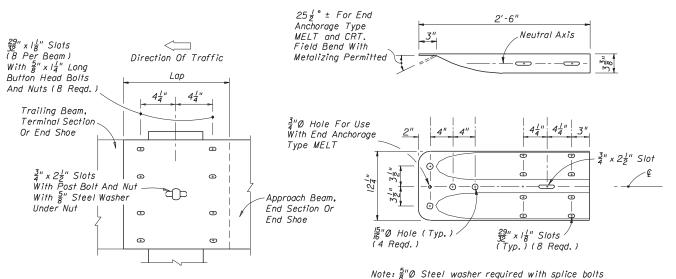
(b) Use of post bolts 15" in length with the washers and nuts counter sunk into sinks I" to 12" deep or

(c) Use of post bolts 15" in length with sleeve nuts and washers.

3. The cost for Special Safety Pipe Rail, mounting components and installation shall be included in the contract unit price for Guardrail.

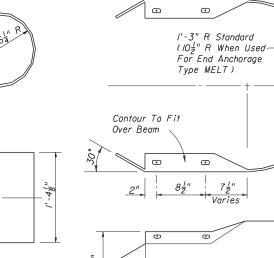
FOR LOCATIONS USED BY PEDESTRIANS OR CYCLISTS

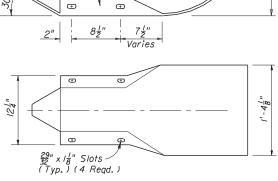




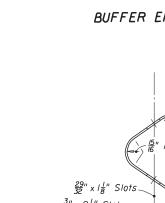
$\frac{3}{4}$ " x $2\frac{1}{2}$ " Slot $\frac{29}{32}$ " x l_8^{1} " Slots (Typ.) (8 Reqd.) FLARED END SECTION

 $2' - 3\frac{1}{2}''$





BUFFER END SECTION



3" x 2 = " Slot € Of Beam ⊃ Sheet Thickness ∽Tolerance (-0,+<u>/</u>") 3<u>/</u>"

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

W-BEAM

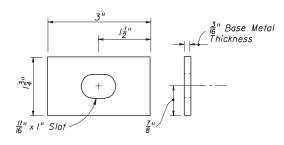
GUARDRAIL

	Names	Dates	Approve	d By	A-71
Designed By				State Roadway	Design Engineer
Drawn By	HSD	8/81	Revision	Sheet No.	Index No.
Checked By	JBW/JVG	8/81	00	18 of 31	400

W-BEAM RAIL SPLICE

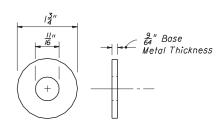
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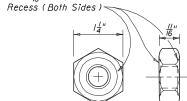
Note: For beam washer requirements on end terminals, see individual end anchorage assembly details. Washers are to be used where necessary to accomplish alignment or where the posts bolt head shows tendency to pull through the rail slot. Washers installed on guardrail, between end anchorages, prior to July 1, 1990 may remain in place until the guardrail is relocated or until repairs require removal and reinstallment of a post bolt.

(RECTANGULAR PLATE WASHER) BEAM WASHER



Note: The round washer is not intended for use under the recess nut for the beam to beam rail splice. The washer is required under the recess nut for connecting the beam to the special end shoe; under the post bolt nut for connecting the beam to the timber post and offset blocks; for connecting the beam to steel posts with timber offset blocks; under the hex bolt head for securing the beam anchor plate to the beam; and, for general guardrail connections by \(\frac{8}{6}\) \(\text{M} \) hex bolts and nuts. For supplemental information see BEAM ANCHOR PLATE, PERMISSIBLE POST AND OFFSET BLOCK COMBINATIONS, individual end anchorage assembly details, SPECIAL STEEL GUARDRAIL POSTS, SPECIAL END SHOE, W-BEAM RAIL SPLICE, THRIE-BEAM RAIL SPLICE, and THRIE-BEAM TERMINAL CONNECTOR details.

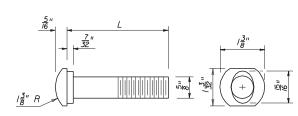
₹" STEEL WASHER



SPECIAL END SHOE

 $I''Ø \times \frac{I}{16}''$ Deep

를" MODIFIED HEAVY HEX NUT (RECESSED NUT)



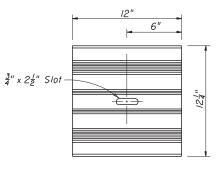
L (In.)	THREAD LENGTH (Min.)(In.)	APPLICATION
/ <u>/</u> "	Full Length	Rail Splice Bolt
10"	4"	Single Or Double Faced Guardrail Post Bolt – Timber Or Recycled Plastic Offset Block(s) On Steel Post
18"	4"	Post Bolt - Single Faced Guardrail Timber Posts
25"*	4"	Post Bolt - Double Faced Guardrail Timber Posts

Special bolts having lengths of IO" or greater shall have a thread length of not less than 4".

* Use of the 25" AASHTO-AGC-ARTBA standard length post bolt on double faced quardrail that results in the bolt projecting more than $\frac{3}{4}$ beyond the face of the nut after pull-up shall be trimmed to $\frac{3}{4}$ reveal and metalized with organic zinc-rich coating.

Note: Specifications same as for hex bolts.

OVAL SHOULDER BUTTON HEAD BOLT



Note: For application information see individual end anchorage assembly details.

W-BEAM BACK-UP PLATE

Hex bolts shall conform to the requirements of ASTM F568M and hex nuts to the requirements of ASTM A563M. Heavy hex nuts may be used in lieu of hex nuts and hex nuts used for jam nuts.

Contour To Fit

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 $\frac{29}{32}$ " x l_8^{1} " Slots (Typ.) (4 Reqd.)

ROUNDED END SECTION

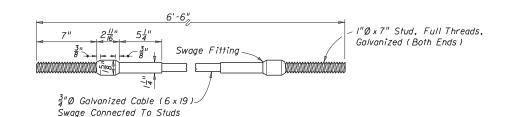
Over Beam

HEX BOLTS AND NUTS

OFFSETS (Ft.) Measured From Face Of Guardrail To Front Of Above Ground Rigid Hazard							
POST	SINGLE	BEAM	NESTED BEAMS				
SPACING (Ft.)	W -Beam	Thrie-Beam	W -Beam	Thrie-Beam			
6'-3"	4'	3'-3"	N/A	N/A			
3'-1½"	3' 2'-8"		2'-8"	2'-4"			
1'-6 3 "	N/A	N/A	2'-4"	2'			

Note: The values shown should be utilized unless changes are supported by imperical validation. Those desiring to develop offset values from the simulated deflection values shown in Table 5.3 of the AASHTO Roadside Design Guide are cautioned to procede only if background in the table development is understood.

> MINIMUM OFFSET FOR SINGLE FACED GUARDRAIL (Ft.)

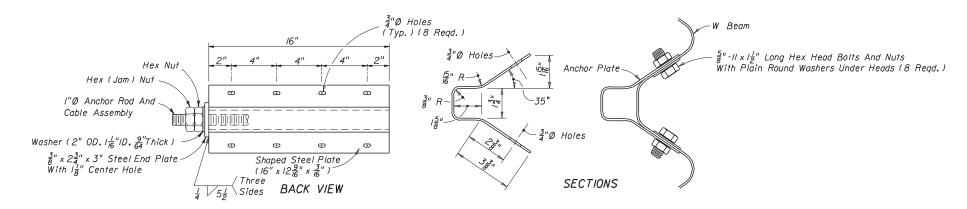


Note: Cable assemblies shall be in accordance with the specifications of AASHTO-AGC-ARTBA 'A Guide To Standardized Highway Barrier Hardware' Cable Anchor Assembly FCAOI. Cable assemblies as detailed above are required as a part of end anchorage assemblies Type II (Cable Option), MELT, SRT -350, CRT, ET -2000, BEST, LET, SKT -350, FLEAT -350 and REGENT. An additional cable assembly 9 in length with a swaged fitting on one (I) end is required for each end anchorage assembly Type CRT.

CABLE ASSEMBLY

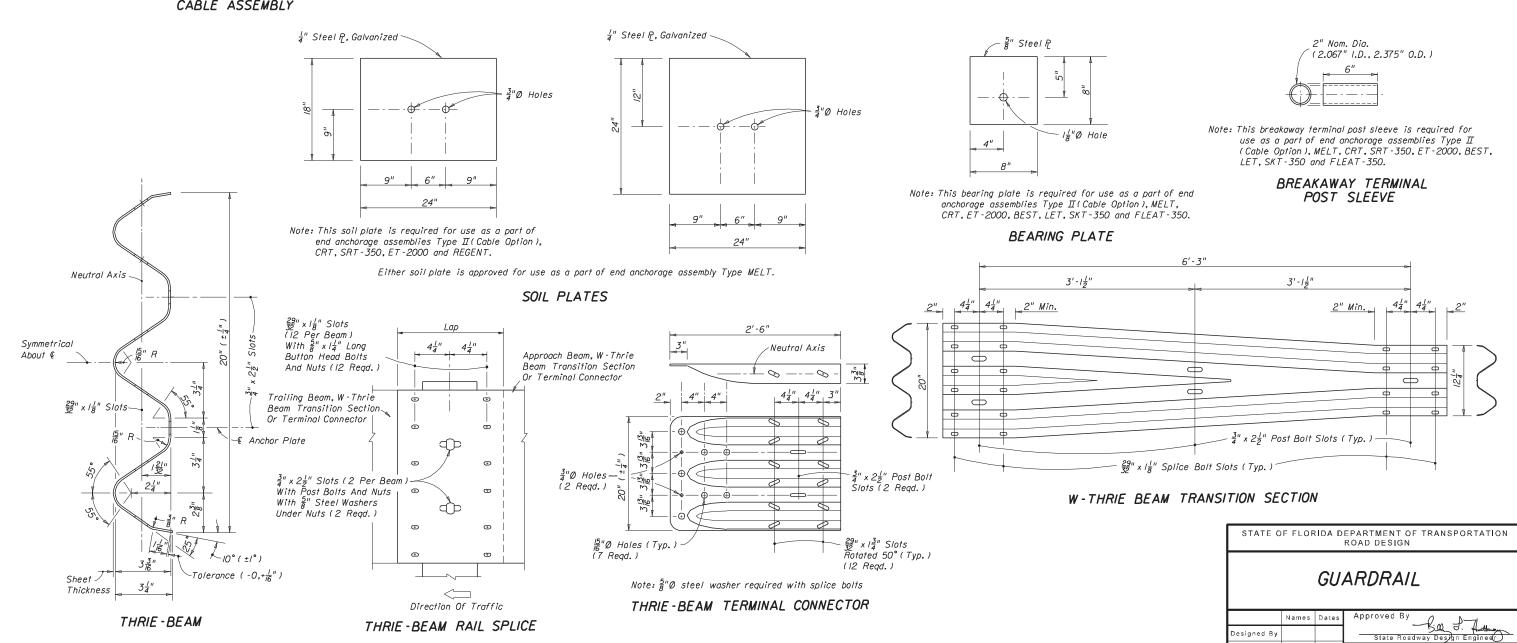
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Note: Beam anchor plates are required for use as a part of end anchorage assemblies Type II, MELT, SRT-350, CRT and REGENT.

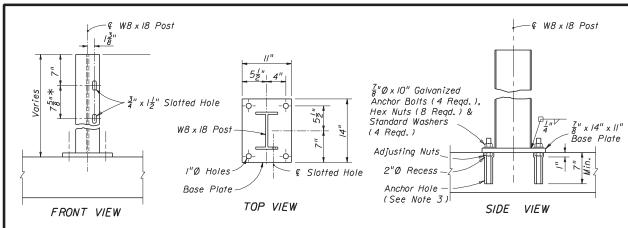
BEAM ANCHOR PLATE



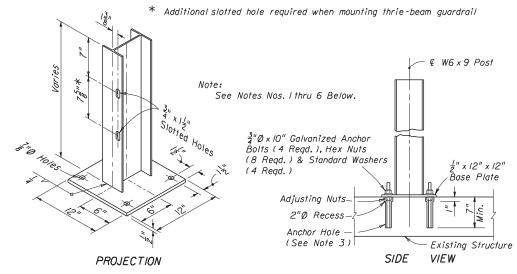
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FOR MOUNTING GUARDRAIL ON EXISTING APPROACH SLABS AND BRIDGE SIDEWALKS



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FOR CONSTRUCTION OF GUARDRAIL WHERE CULVERT, PIER FOOTING OR OTHER STRUCTURE PRECLUDES NORMAL POST INSTALLATION

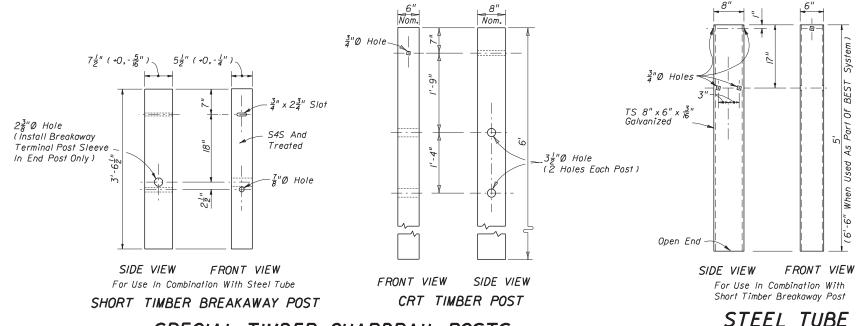
NOTES: (SPECIAL STEEL POST)

 Either anchor bolts, concrete wedge anchors or approved Adhesive-Bonded Anchors for Structural Applications may be used.

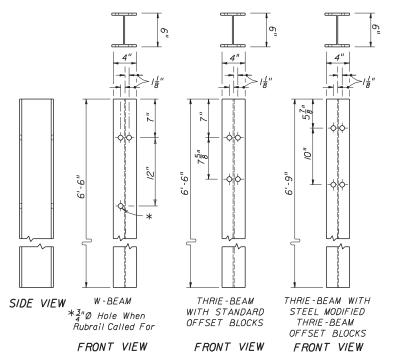
Anchor bolts, wedge anchors and adhesive anchors shall have a minimum tensile strength of 60,000 psi and galvanized in accordance with ASTM A/53 (stainless steel componets may be substituted but components plated in accordance with ASTM B-633 are not acceptable). Adhesive anchor rods shall be equal in diameter to that detailed for anchor bolts. Wedge anchors are to be installed in accordance with the manufacturers recommendations, assuming 3,000 psi compressive strength for concrete. Wedge anchors shall also meet the following requirements: (a) tensile load each anchor: approach slabs 14,000 lbs.; other structures 8,000 lbs. (b) shear load each anchor: approach slabs 15,000 lbs.; other structurers 7,800 lbs.

- 2. Posts are to be plumbed by adjusting nuts or mortar seating. Posts installed using anchor bolts and adhesive anchors are to be set with adjusting nuts as detailed, unless the Engineer approves the use of mortar seating in lieu of adjusting nuts. Posts installed using wedge anchors are to be set with mortar seating. Base plates shall be grouted with neat finish.
- 3. Adhesive-Bonded Anchors for Structural Applications shall comply with Section 937 and be installed in accordance with Section 416. Drilled hole diameter shall be in accordance with the manufacturer's instructions.
- 4. Anchor holes and recesses shall be drilled; wedge anchor holes are to be drilled in accordance with the manufacturers specifications. Encountered reinforcing steel shall be drilled through. Holes shall be thoroughly cleaned when setting bolts and anchors and dry when setting wedge anchors.
- 6. Steel post and base units shall be galvanized in accordance with ASTM A/23. Any damaged galvanized areas are to be metalized in accordance with Section 562 of the Standard Specifications.

SPECIAL STEEL GUARDRAIL POSTS



SPECIAL TIMBER GUARDRAIL POSTS



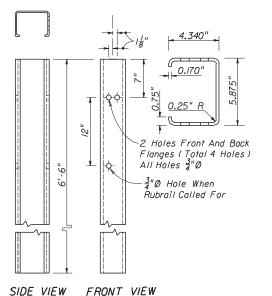
All Holes Shall Be $\frac{3}{4}$ " Ø Identical Front And Back Flanges

Note: W6 x 8.5 or W6 x 9 steel posts may be either rolled or welded structural shapes conforming to or exceeding the design properties of ASTM A6/A6M. Welding shall be in accordance with the requirements of ASTM A769/A769M. Posts shall be cut to length and the ends seal welded between web and flange before galvanizing. Posts to be galvanized in accordance with ASTM A/23.

W6 x 8.5 OR W6 x 9 STEEL POST

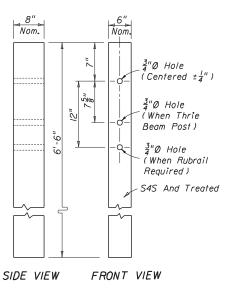
STANDARD TIMBER AND STEEL GUARDRAIL POSTS

GUARDRAIL POSTS



Note: 6"-C steel posts are to face the same direction in any continuous run of guardrail. Posts to be galvanized in accordance with ASTM Al23.

6"-C STEEL POST

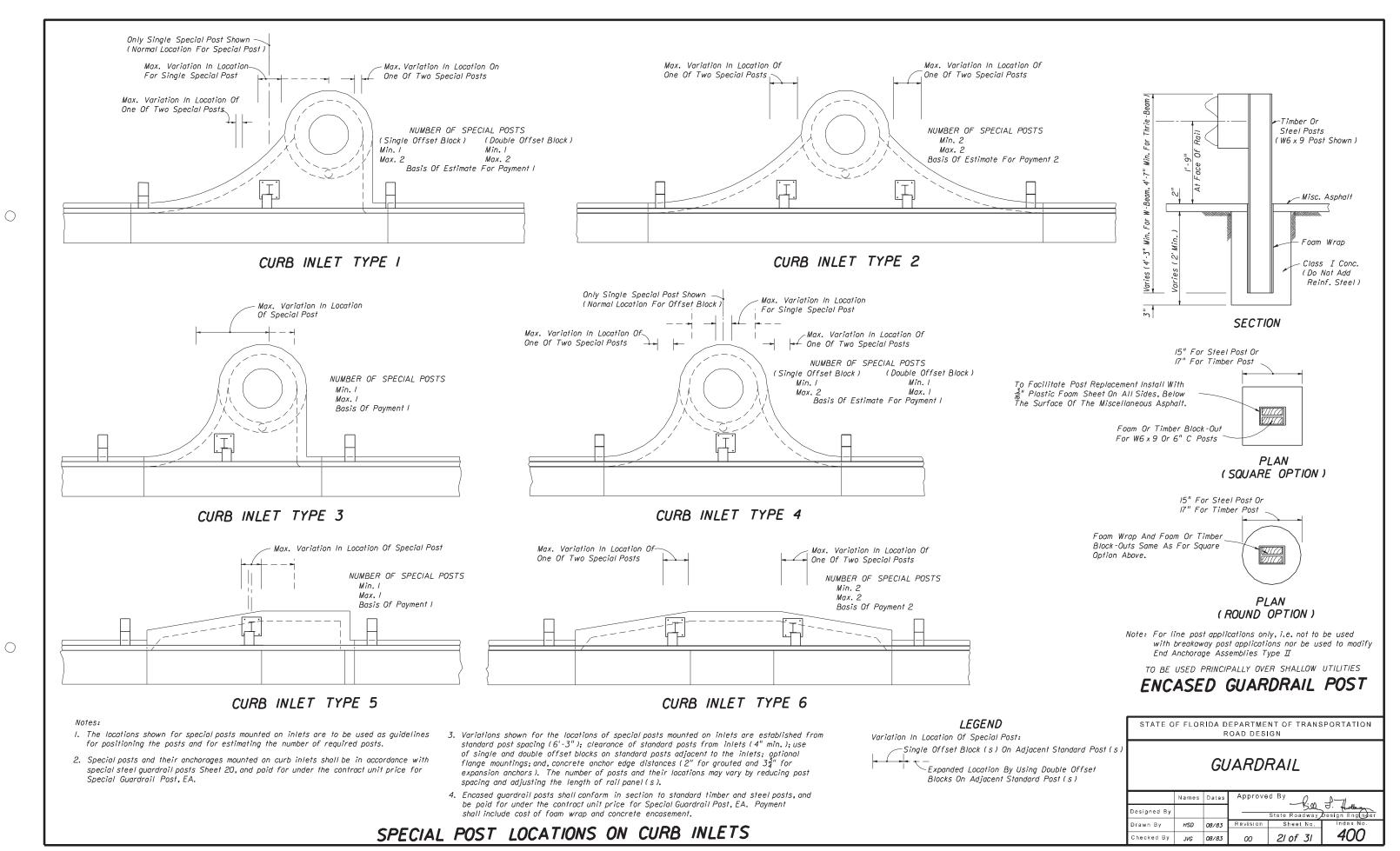


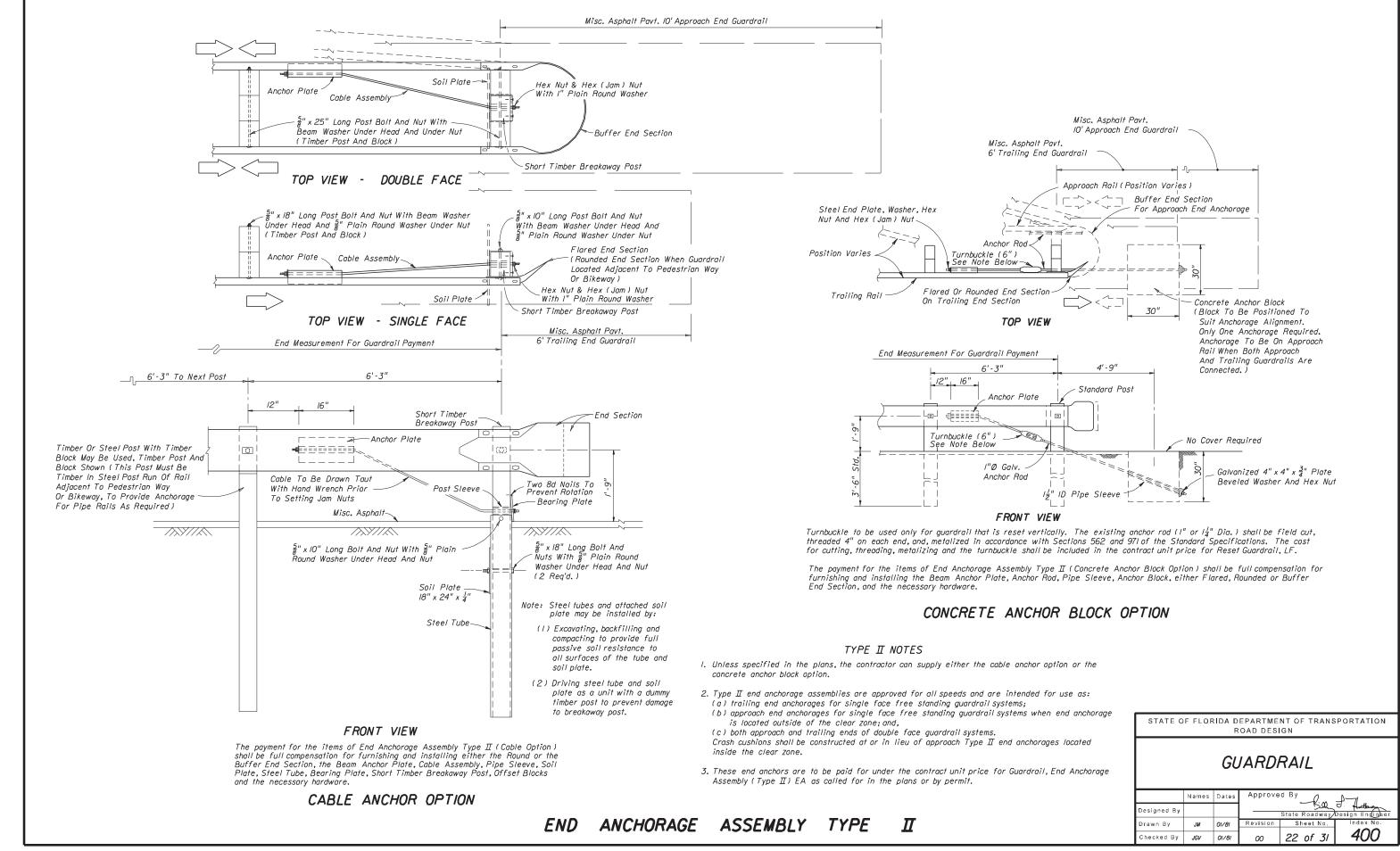
TIMBER POST

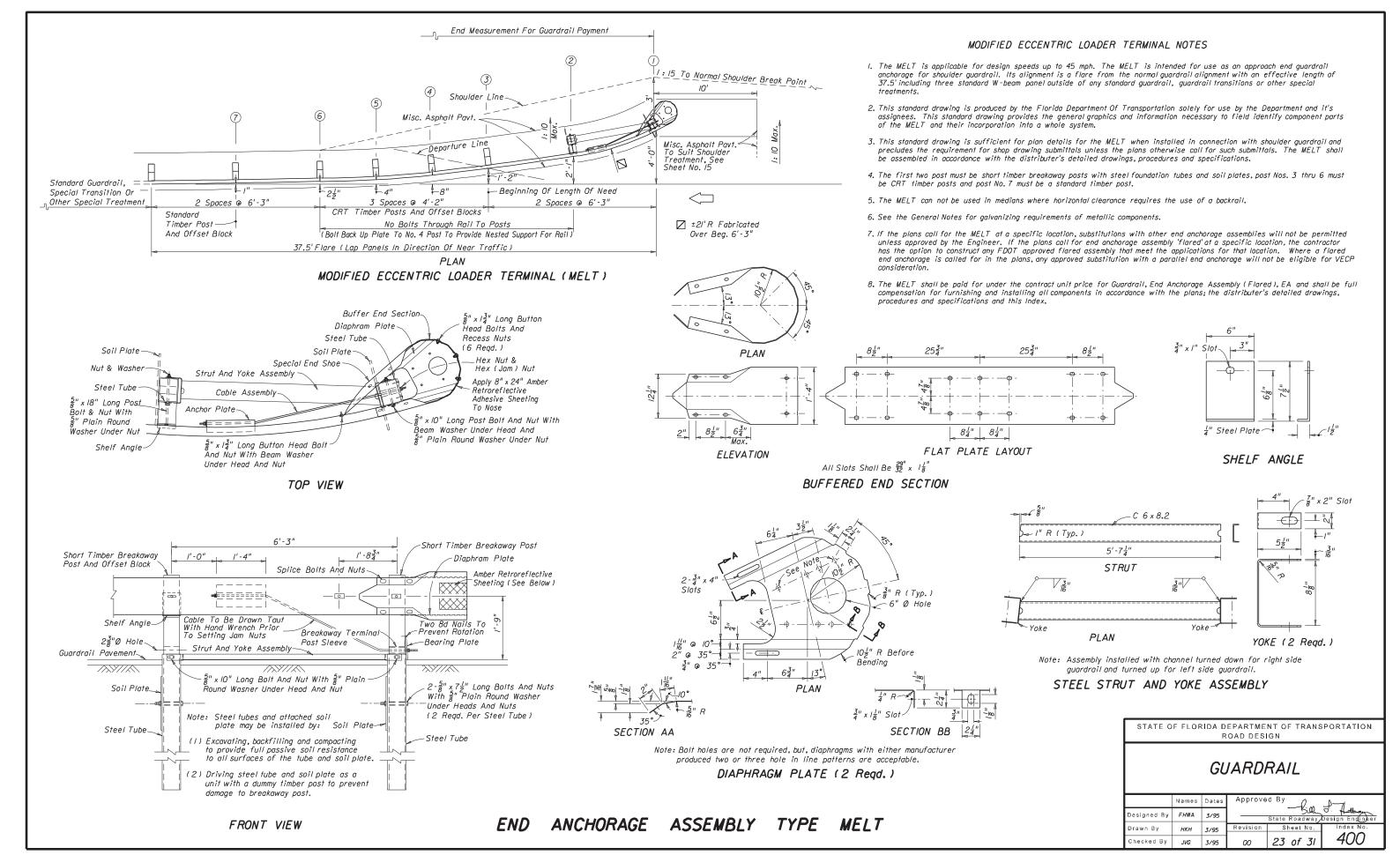
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

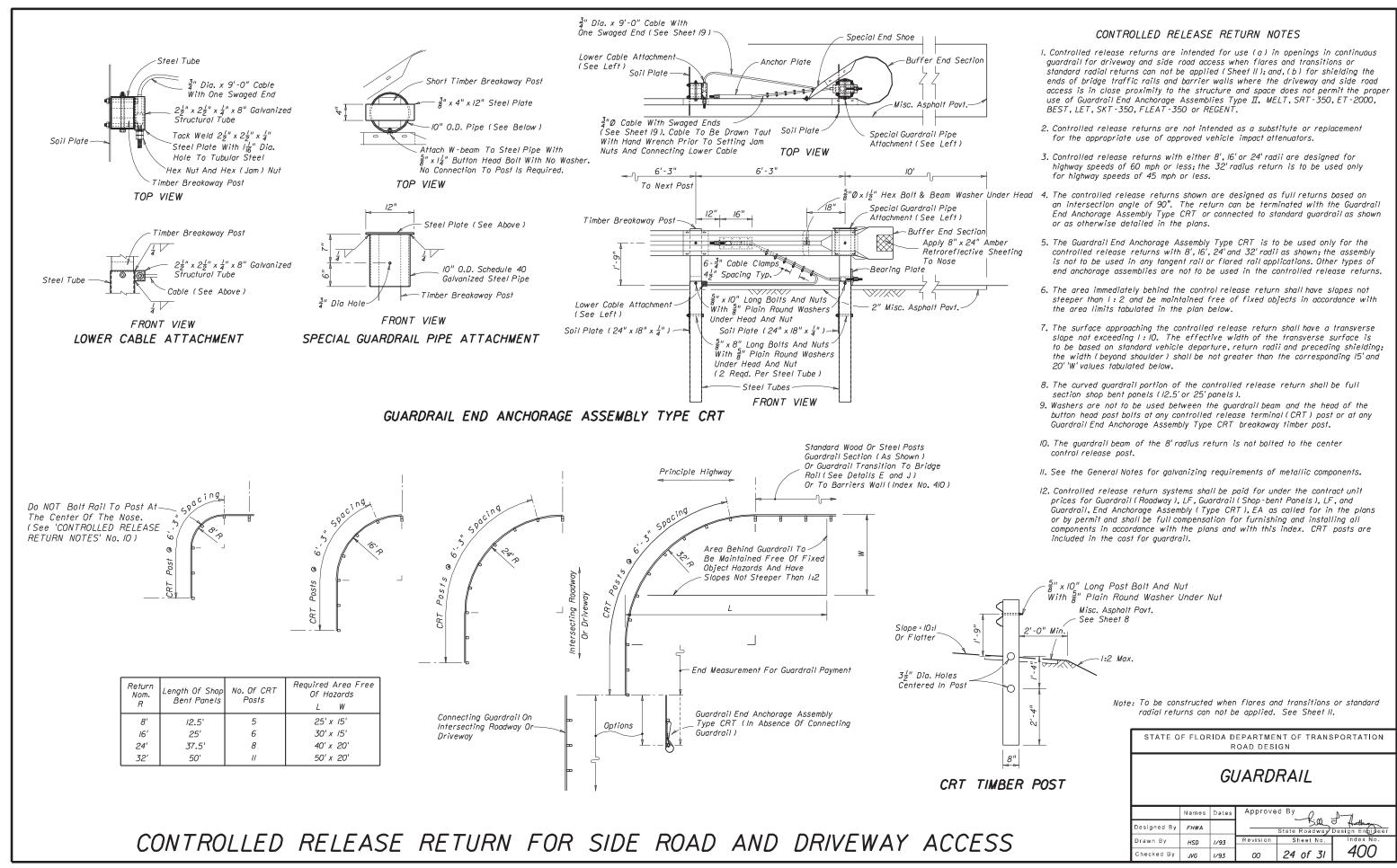
GUARDRAIL

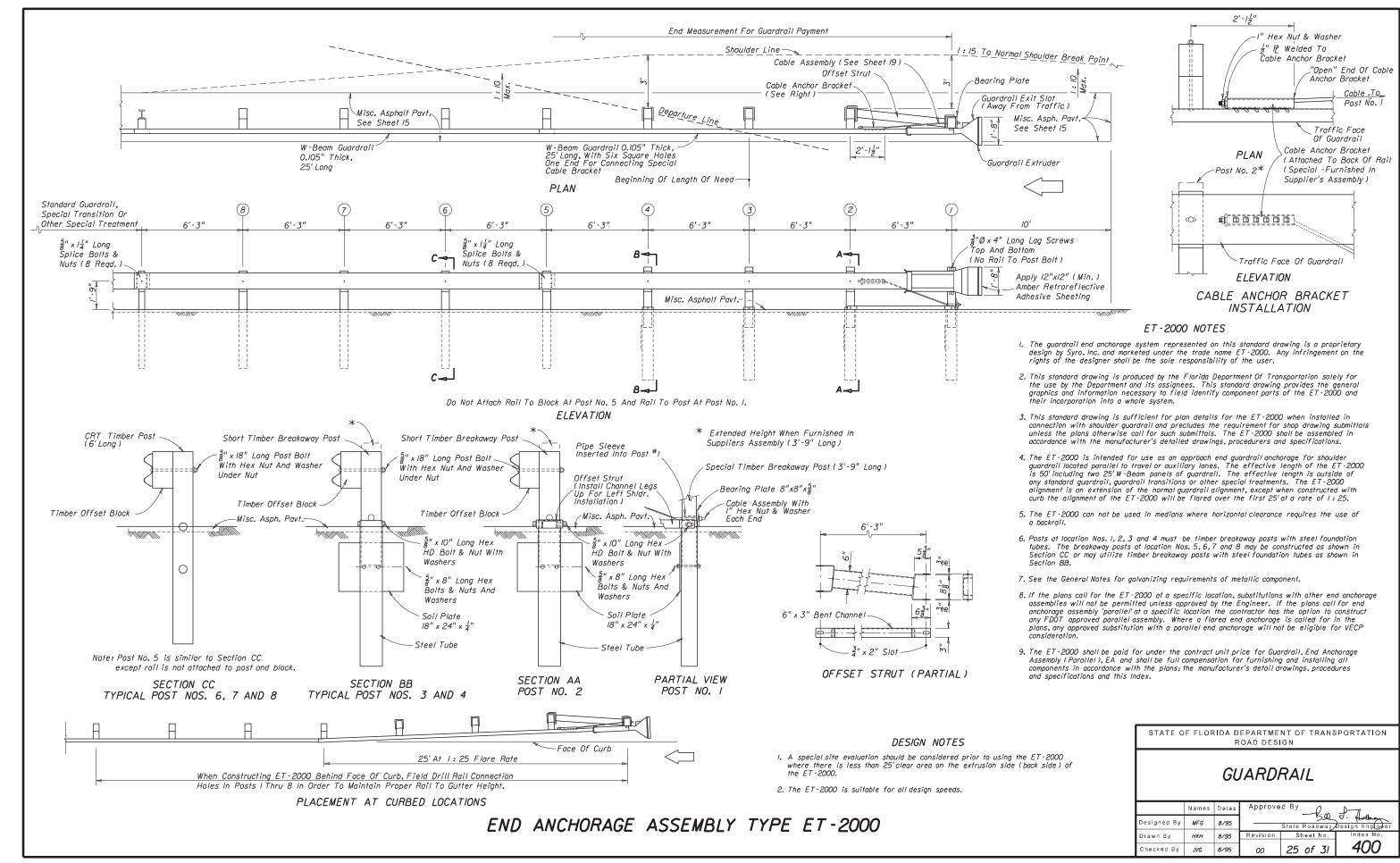
	Names	Dates	Approve	d By	♪ ──/
Designed By				State Roadway	Design Engineer
Drawn By	JM	08/81	Revision	Sheet No.	Index No.
Checked By	JVG/JBW	08/81	00	20 of 31	400

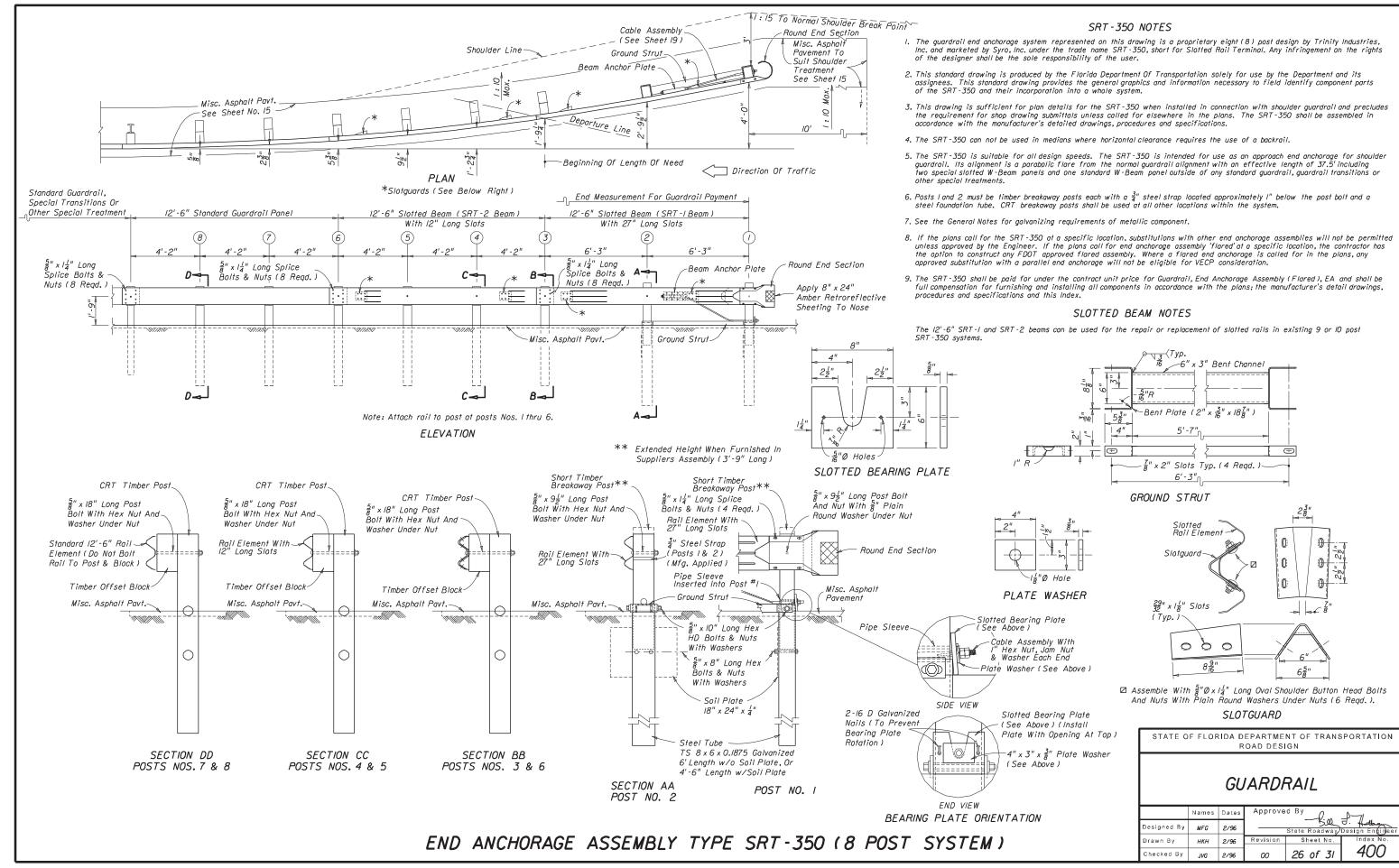


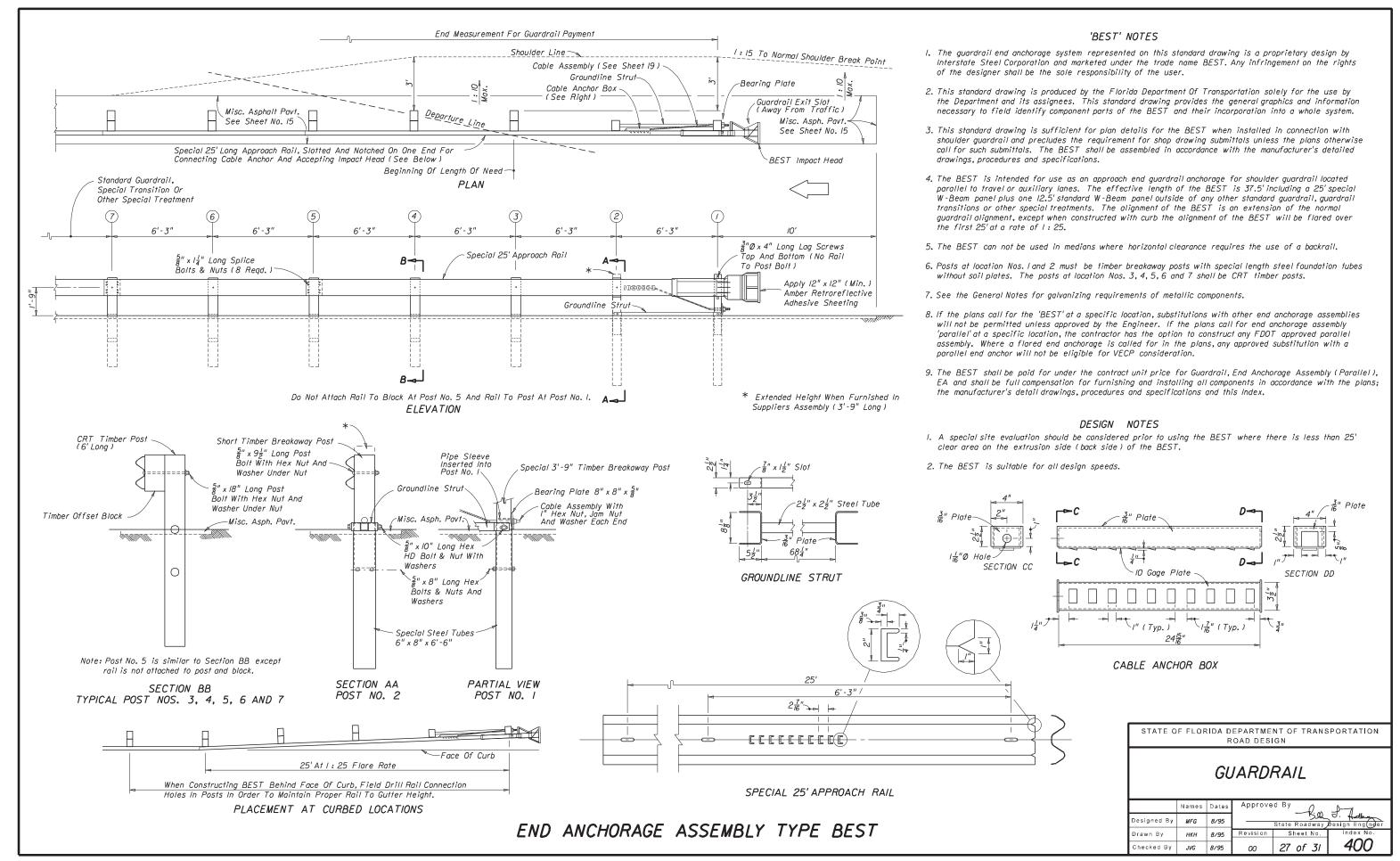


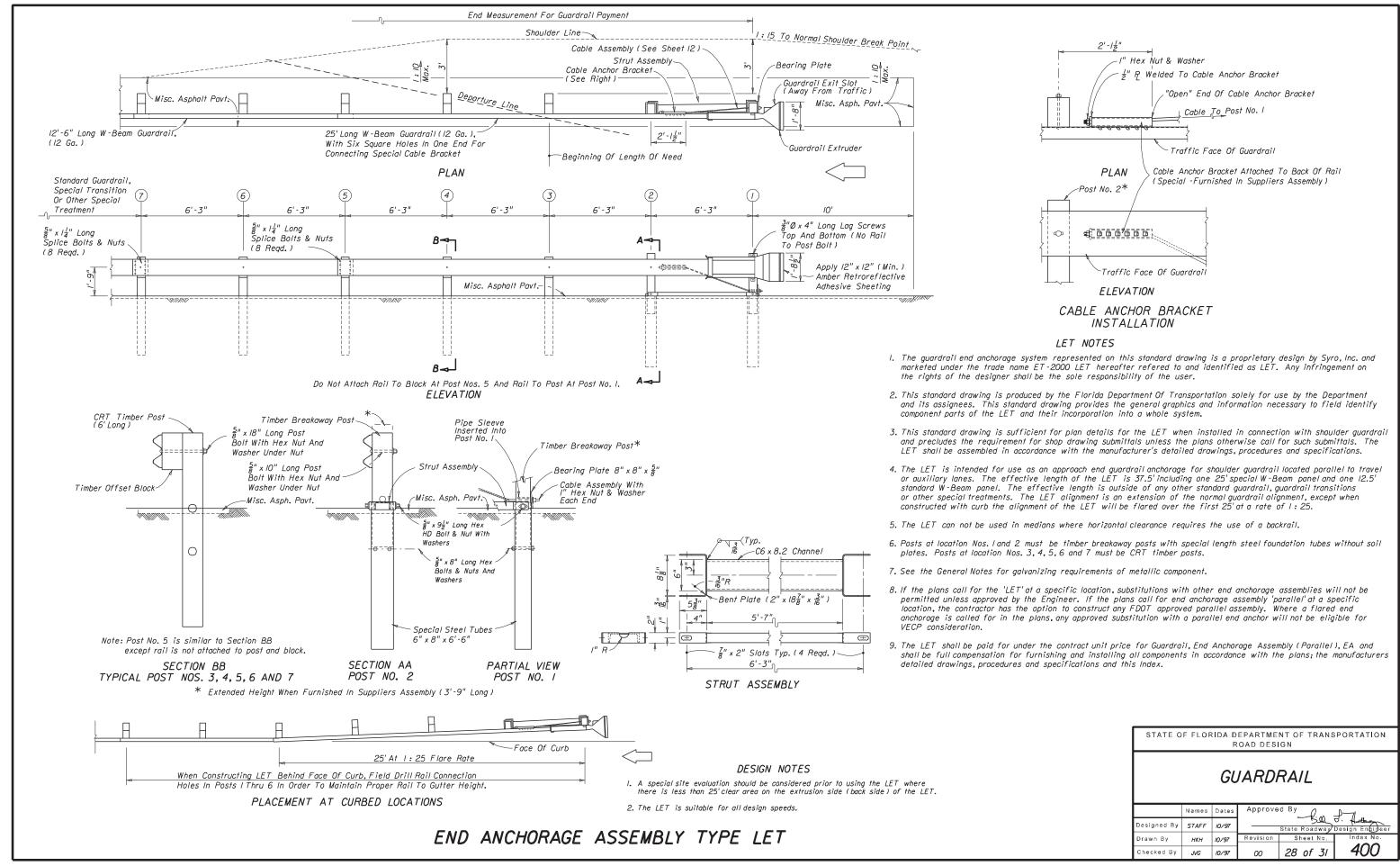


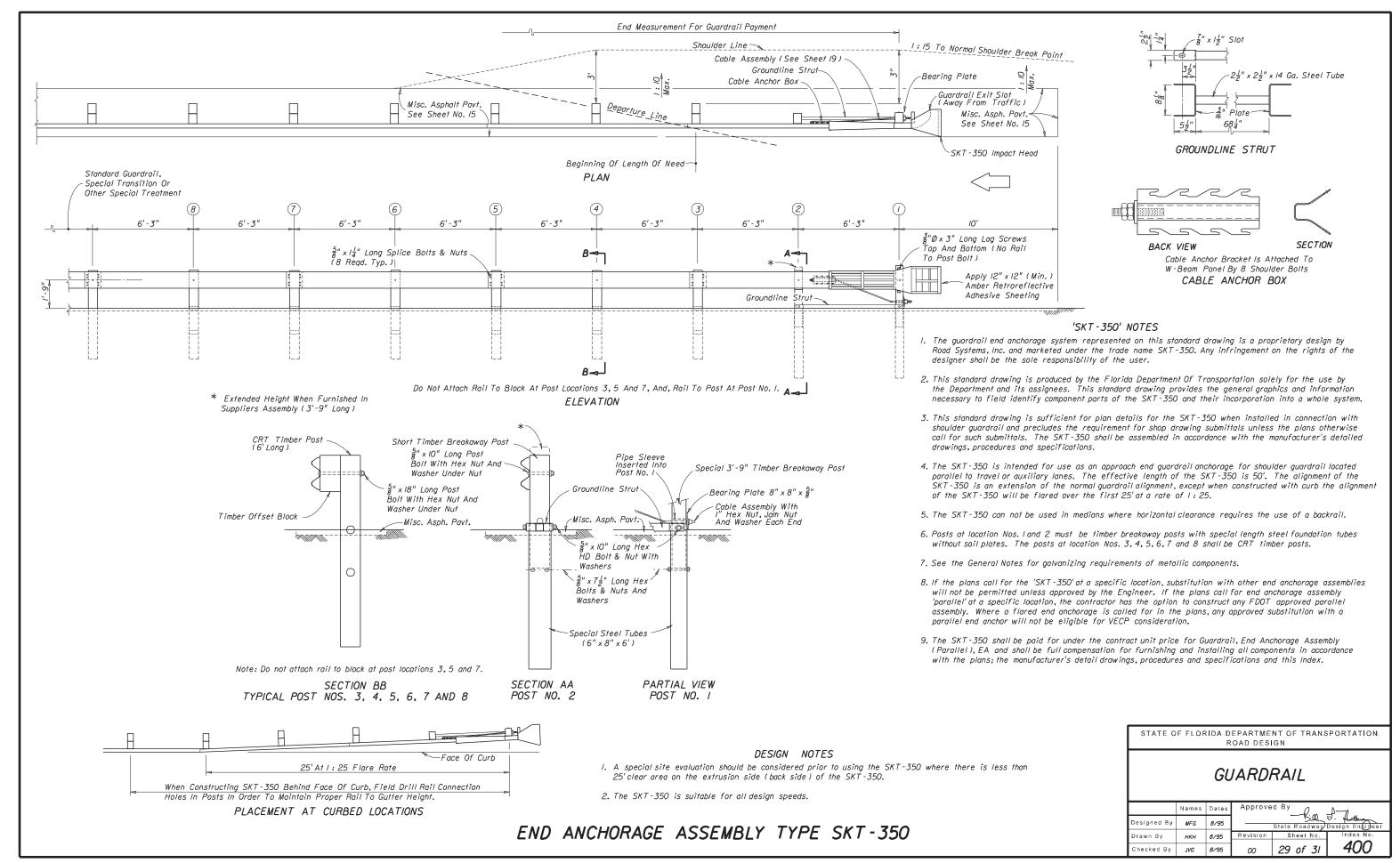


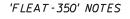








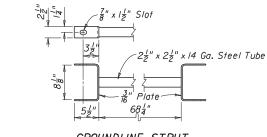




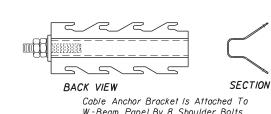
- I. The guardrail end anchorage system represented on this standard drawing is a proprietary design by Road Systems, Inc. and marketed under the trade name FLEAT-350. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for the use by the Department and its assignees. This standard drawing provides the general graphics and information necessary to field identify component parts of the FLEAT-350 and their incorporation into a whole system.
- 3. This standard drawing is sufficient for plan details for the FLEAT-350 when installed in connection with shoulder guardrail and precludes the requirement for shop drawing submittals unless the plans otherwise call for such submittals. The FLEAT-350 shall be assembled in accordance with the manufacturer's detailed drawings, procedures and specifications.
- 4. The FLEAT-350 is intended for use as an approach end guardrail anchorage for shoulder guardrail on one-way facilities and divided multilane facilities. The effective length of the FLEAT-350 is 37.5' including one 12.5' special W-Beam panel plus two 12.5' standard W-Beam panels outside of any other standard quardrail, quardrail transitions or other special treatments. The alignment of the FLEAT-350 is a straight flare with end offset of 4' from the normal guardrail alignment.
- 5. The FLEAT-350 can not be used in medians where horizontal clearance requires the use of a backrail.
- 6. Posts at location Nos. Land 2 must be timber breakaway posts with special length steel foundation tubes without soil plates. The posts at location Nos. 3, 4, 5, 6, and 7 shall be CRT timber posts.
- 7. See the General Notes for galvanizing requirements of metallic components.
- 8. If the plans call for the 'FLEAT-350' at a specific location, substitution with other end anchorage assemblies will not be permitted unless approved by the Engineer . If the plans call for end anchorage assembly 'flared' at a specific location, the contractor has the option to construct any FDOT approved flared assembly. Where a flared end anchorage is called for in the plans, any approved substitution with a parallel end anchorage will not be eligible for VECP consideration.
- 9. The FLEAT-350 shall be paid for under the contract unit price for Guardrail, End Anchorage Assembly (Flared), EA and shall be full compensation for furnishing and installing all components in accordance with the plans; the manufacturer's detail drawings, procedures and specifications and this Index.

DESIGN NOTES

I. The FLEAT-350 is suitable for all design speeds.



GROUNDLINE STRUT



W-Beam Panel By 8 Shoulder Bolts CABLE ANCHOR BOX

END ANCHORAGE ASSEMBLY TYPE FLEAT-350

1:15 To Normal Shoulder Break Point

Misc. Asph. Pavt.

See Sheet No. 15

FLEAT 350 Impact Head

 $\frac{3}{8}$ " Ø x 3" Long Lag Screws

(2 Regd.) (No Rail To

Guardrail Exit Slot (Traffic Side)

TRAFFIC

10'

Apply 12" x 12" (Min.)

Amber Retroreflective Adhesive Sheeting

Post Bolt)

End Measurement For Guardrail Payment

Special 12'-6" W-Beam Rail Element-

No Rail To Post Attachment At Post #3

PLAN

No Rail To Post Attachment At Post #3 **ELEVATION**

Groundline Strut

Misc. Asph. Payt.

Washers

景" x IO" Long Hex HD Bolt & Nut With

5" x 7년" Long Hex-Bolt & Nut And Washers (I Regd. Per Post)

Special Steel Tubes

(8" x 6" x 1/8", 6' Long

Pipe Sleeve

Inserted Into

37'-6" Straight Flare

 $\frac{1}{8}$ " x l_4 " Long Splice Bolts & Nuts

(8 Read. Typ.)

B

B⊸

W///////////

SECTION AA

POST NO. 2

Short Timber Breakaway Post 5" x 10" Long Post

Washer Under Nut

x 18" Long Post

Washer Under Nut

Bolt With Hex Nut And

-Misc. Asph. Pavt.

Bolt With Hex Nut And

Standard Guardrail, Special Transition or Other Special Treatment

Standard I2'-6" Rails

6'-3"

CRT Timber Post (6' Long)

101111111111 - I

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Note: Do not attach rail to block at post locations 3, 5 and 7.

SECTION BB

TYPICAL POST NOS. 3, 4, 5, 6 AND 7

Timber Offset Block

Cable Assembly (See Sheet 19)

Groundline Strut

Beginning Of Length Of Need

Special 12'-6" W-Beam Rail Element

Groundline Strut-

Timber Breakaway Post*

7/1/////////

PARTIAL VIEW

POST NO. I

Bearing Plate 8" x 8" x \frac{5}{8}"

Cable Assembly With

I" Hex Nut, Jam Nut And Washer Each End

* Extended Height When Furnished In

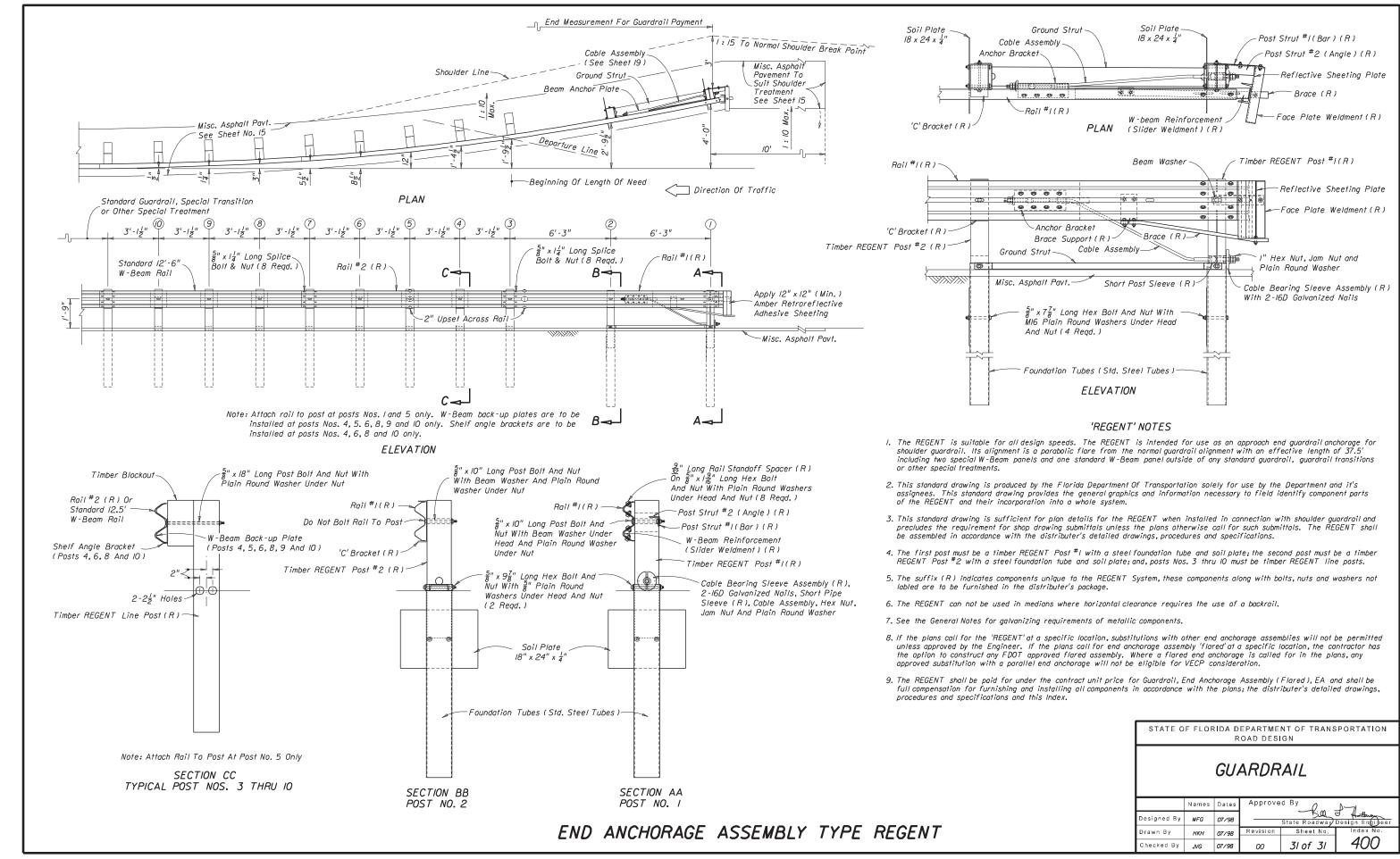
Suppliers Assembly (3'-9" Long Post)

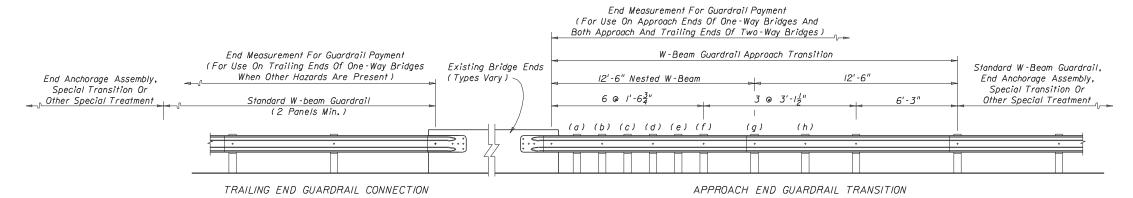
Cable Anchor Box

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

GUARDRAIL

	Names	Dates	Approve	d By	♪ /1
Designed By	MFG	07/98		State Roadway	Design Engineer
Drawn By	нкн	07/98	Revision	Sheet No.	Index No.
Checked By	JVG	07/98	00	30 of 31	400





ELEVATION

GENERAL NOTES

I. Whether an existing bridge handrail is to remain in place, be retrofitted or be replaced, is a determination that must be made independent of any information contained on this Index.

Only after it has been established that an existing bridge handrail is to remain in place is this Index to be used to analyze guardrail to bridge connections.

- 2. The schemes on this index are not to be used for new bridge construction, bridge widening, bridge barrier wall or handrail replacement, or, for existing bridges that have safety shape traffic barrier.
- 3. The schemes on this index are divided into two general categories, representing curbed and uncurbed roadway approaches. A scheme selection guide is provided below for curbed and uncurbed roadway approaches. Approach slabs with curbs or wing walls with radial safety curbs will be treated as curbed roadway approaches.
- 4. Existing bridge features shown in these schemes are example configurations only. The principle key to scheme selection is bridge curb or sidewalk width. Location control is keyed to bridge face of curb, except for certain trailing conditions.
- Details that are repetitive on the schemes and features that are detailed on Index No. 400 have been purposely deleted to produce clarity and simplification in the schemes, and to emphasize proper location and positioning of the anchorage and connecting guardrail.
- 6. All schemes are right side or right hand details for traffic flow right to left. Left side applications are opposite hand.
- 7. For undivided two-way bridges 'trailing end', as used in this index, is in relation to the direction of travel of near lane traffic, but it is always considered as an approach for opposing lane traffic.
- 8. All connections of guardrail special end shoes to concrete anchorage posts, panels and walls shall have a $12'' \times 12'' \times \frac{5}{8}''$ galvanized steel back-up plate for gang tightening of hex nuts on $\frac{7}{8}''$ diameter galvanized hex bolts; hex bolts shall have a nominal length equal to the thickness of the concrete anchorage plus $1\frac{1}{2}''$.

When thru bolts would penetrate existing bridge rails, $\frac{7}{8}$ diameter bolt clusters and chemical anchor bolts meeting the manufacturers recommendation may be substituted as approved by the Engineer.

- Unless otherwise called for in the plans exposed concrete surfaces shall have a Class 3 surface
 finish and Class 5 Applied Finish Coating in accordance with Section 52I and 400 respectively of
 the Standard Specifications.
- 10. The guardrail end anchorage schemes on this index do not include cost for payment of guardrail. See above for limit of guardrail measurement.

Each independent anchorage described in these schemes shall be paid for as a bridge end anchorage assembly under the contract unit price for Bridge Anchorage Assembly, EA.

The unit price shall be full compensation for the following:

- (a) Each concrete anchor post, panel or transition wall including reinforcing steel, existing rail or rail and post removal, socket filling, bond breaker, post beveling, drilling, dowels, grouting, excavation, backfill, special end shoe and accessory items.
- (b) Each guardrail steel terminal post, including flared end section, anchorage and accessory items (optional use not included).
- (c) Each special end shoe anchored directly to an existing bridge end post or wing post, including back-up plate and accessory items.

Continuous concrete safety barrier (Schemes I & I9) shall be paid for as a roadway item under the contract unit price for Concrete Handrail (Retrofit Barrier) (Vert. Face), LF.

Continuous guardrail across bridges shall be paid for as a roadway item under the contract unit price for Guardrail (Bridge) LF, and Special Guardrail Post, EA. The unit price for guardrail shall include the cost for all accessories prescribed under Index No. 400 and the unit price for special posts shall include the cost for all accessories and anchorage prescribed in Index 400 and in Scheme I6 of this Index.

GUARDRAIL NOTE

Approach end guardrail transitions shall be constructed on the approach ends of one-way bridges, and both the approach and trailing ends of undivided two-way bridges. Nested rails shall not be bolted to the blocks and posts at posts (a),(c), and (e). Trailing end guardrail connections shall be constructed on the trailing ends of one way bridges when other hazards are present. One I6d galvanized nail shall be driven between each timber post and block, and between double blocks, in order to prevent block rotation, see Index No. 400 'I6d NAIL FOR PREVENTION OF OFFSET BLOCK ROTATION'. All posts are standard guardrail posts except where special circumstances require the use of guardrail steel terminal posts, special steel guardrail posts or encased guardrail posts. Refer to Index No. 400 for additional guardrail details.

DESIGN NOTES

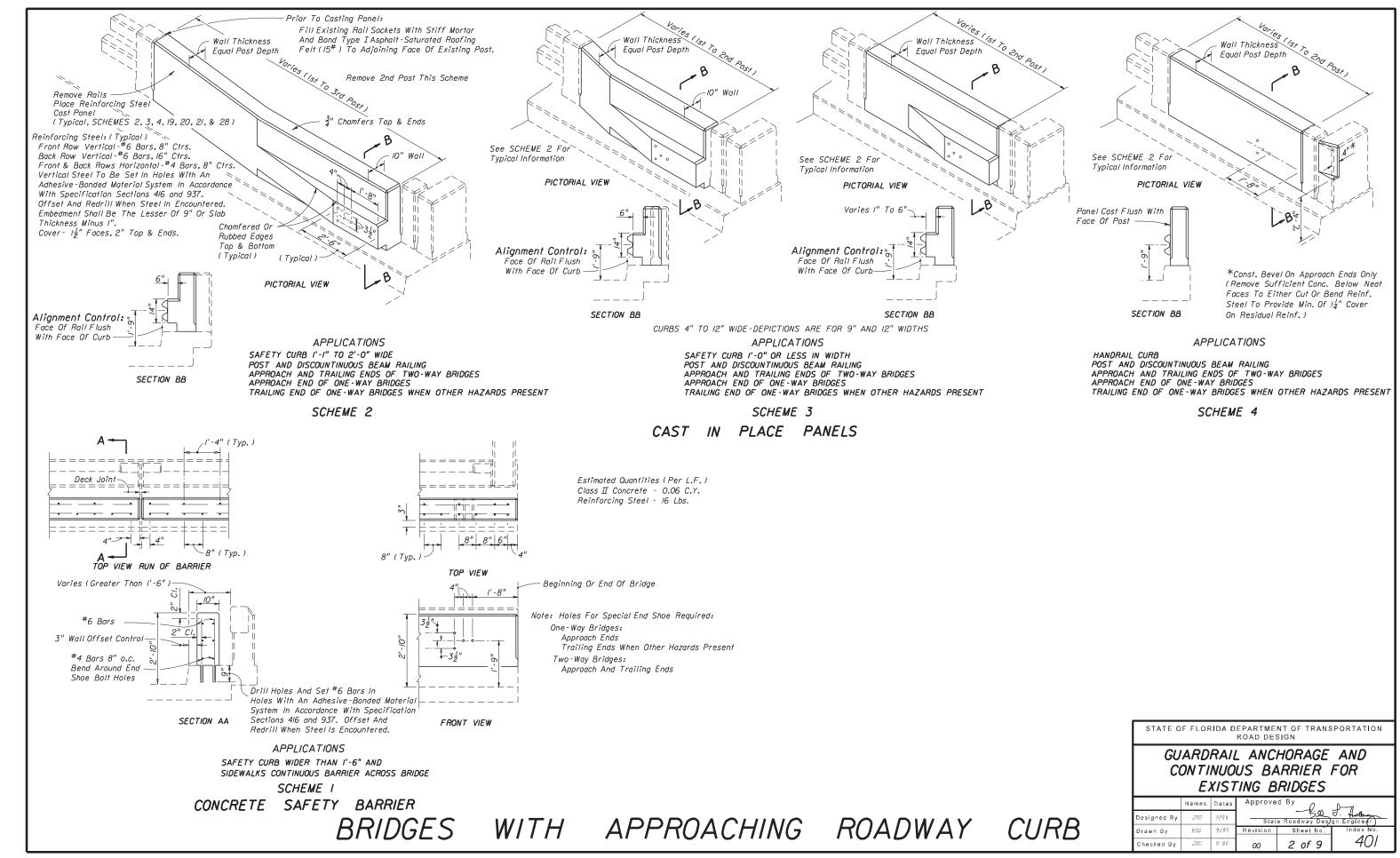
- I. The details in this index are intended to be used for existing bridges that have end and approach slab configurations constructed under former Department standards; and, are not intended to preclude special design details more suited to bridges with unusual handrail or wingpost configurations, or, when there is conflict with drainage structures or other features that can not be adjusted.
- 2. The schemes provide the designer with a convenient method of providing standardized information on the plans. In the selection and assignment of schemes the designer must predetermine existing bridge handrail, curb, sidewalk and approach slab conditions, particularly the location of embedded conduit. Special attention must be directed to the presence or absence of curbed approaches on each independent corner of the bridge.
- 3. Each corner of the bridge that requires a guardrail connection should be labeled independently by scheme number, and, where continuous barrier is required across a bridge the scheme number should be labeled independently on the side(s) of the bridge. When continuous guardrail is called for, bridge end anchorage assemblies will be omitted, but, when continuous concrete safety barrier is called for, one or more bridge end anchorage assemblies will be labeled on the plans.
- 4. The scheme selection guide below is to be used as a quick reference for determining anchorages and continuous barriers that are applicable to specific conditions for existing bridges. When appropriate, special details are to be used in lieu of schemes or to supplement or complement the scheme details. In selecting schemes the width of curb, safety curb and sidewalk is the distance from the face of curb to the nearest face of post, rail or parapet.

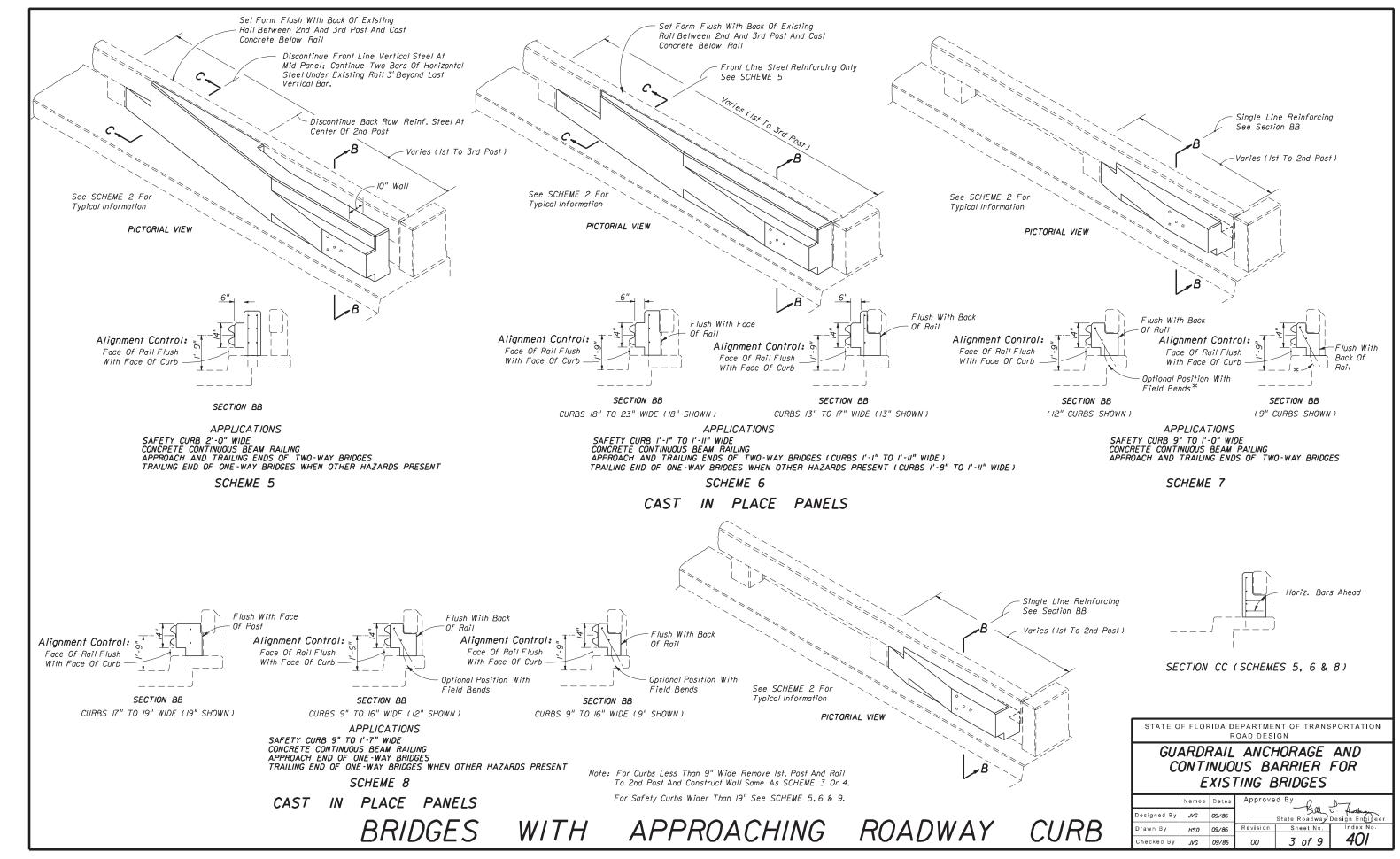
	SCHEME SL	ELECTION GUIDE (NUMBERS)		
		APPROACHING BRIDGES 2 thru 6		S APPROACHING BRIDGES 7 thru 9	
ONE-WAY BRIDGES	APPROACH END	TRAILING END	APPROACH END	TRAILING END	
Handrail Curb	3, 4, 18	3, 4, 18	21, 22, 27, 30	23, 27, 30	
Narrow Curb	2, 3, 8, 9, 10, 11, 12, 13, 14	2, 3, 8, 15	20, 21, 27, 29	23, 27, 29	
Wide Safety Curb	1, 2, 8, 11, 12, 13, 14, 15, 16, 17	1, 2, 8, 11, 12, 13, 14, 15, 16	19, 20, 28, 29	19, 23, 29	
Sidewalks	1, 16	1, 16	19	19	
TWO-WAY BRIDGES	APPROACH AND	TRAILING ENDS	APPROACH AND	TRAILING ENDS	
Handrail Curb	3, 4,	9, 10, 18	21, 22, 26, 30		
Narrow Curb	2, 3, 6, 7, 9,	10, 11, 12, 13, 14	20, 21, 25, 29		
Wide Safety Curb	1, 2, 5, 6, 9, 10,	11, 12, 13, 14, 16	19, 20, 24, 25, 29		
Sidewalks	1,	16	19		

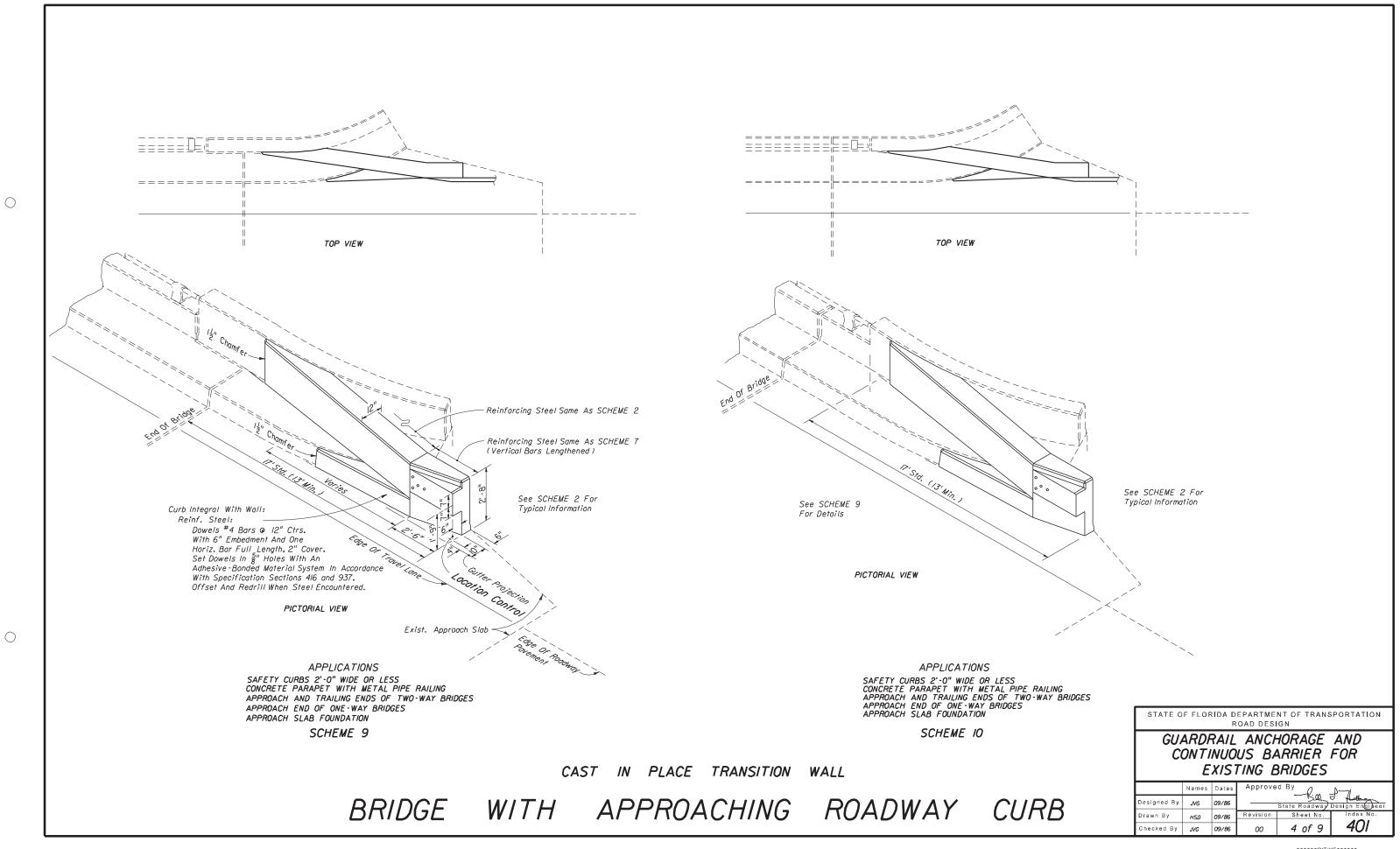
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

GUARDRAIL ANCHORAGE AND CONTINUOUS BARRIER FOR EXISTING BRIDGES

	Names	Dates	Approve	d By	1
Designed By	JVG	09/86		State Roadway	Design Engineer
Drawn By	HSD	09/86	Revision	Sheet No.	Index No.
Checked By	JVG	09/86	00	1 of 9	401







See SCHEME II For Additional Information

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APPLICATIONS

SAFETY CURB 2'-O" WIDE OR LESS APPROACH AND TRAILING ENDS OF TWO-WAY BRIDGES APPROACH END OF ONE-WAY BRIDGES TRAILING END OF ONE-WAY BRIDGES WHEN OTHER HAZARDS PRESENT

SCHEME 12

Exist. Curb PC Or PT

See SCHEMES II & I2 For Additional Information

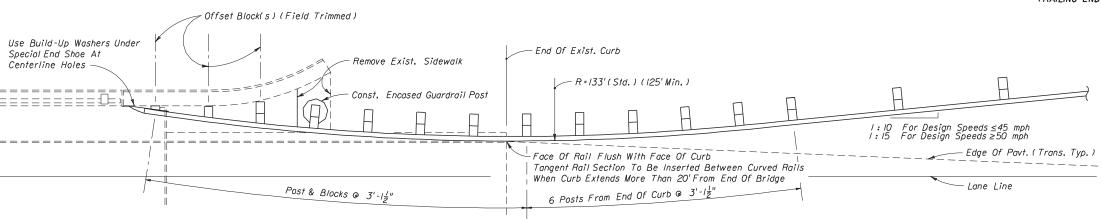
 \sim 6 Posts From End Of Curb @ 3'- l_2^{f} "
Face Of Rail Flush With Face Of Curb At End Of Curb

APPLICATIONS

SAFETY CURB 2'-O" WIDE OR LESS APPROACH AND TRAILING ENDS OF TWO-WAY BRIDGES APPROACH END OF ONE-WAY BRIDGES TRAILING END OF ONE-WAY BRIDGES WHEN OTHER HAZARDS PRESENT

Lane Line

SCHEME 14



APPLICATIONS

SAFETY CURB 2'-0" WIDE OR LESS APPROACH AND TRAILING ENDS OF TWO-WAY BRIDGES APPROACH END OF ONE-WAY BRIDGES TRAILING END OF ONE-WAY BRIDGES WHEN OTHER HAZARDS PRESENT

SCHEME II

CURVILINEAR GUARDRAIL

BRIDGES WITH APPROACHING ROADWAY CURB

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

GUARDRAIL ANCHORAGE AND CONTINUOUS BARRIER FOR EXISTING BRIDGES

Portions Of Curb And Gutter May Be Removed And Reconstructed As Necessary To Locate Posts In This Zone. Post May Be Located Within The Curb Or Within The Lip Of The Gutter, But Not

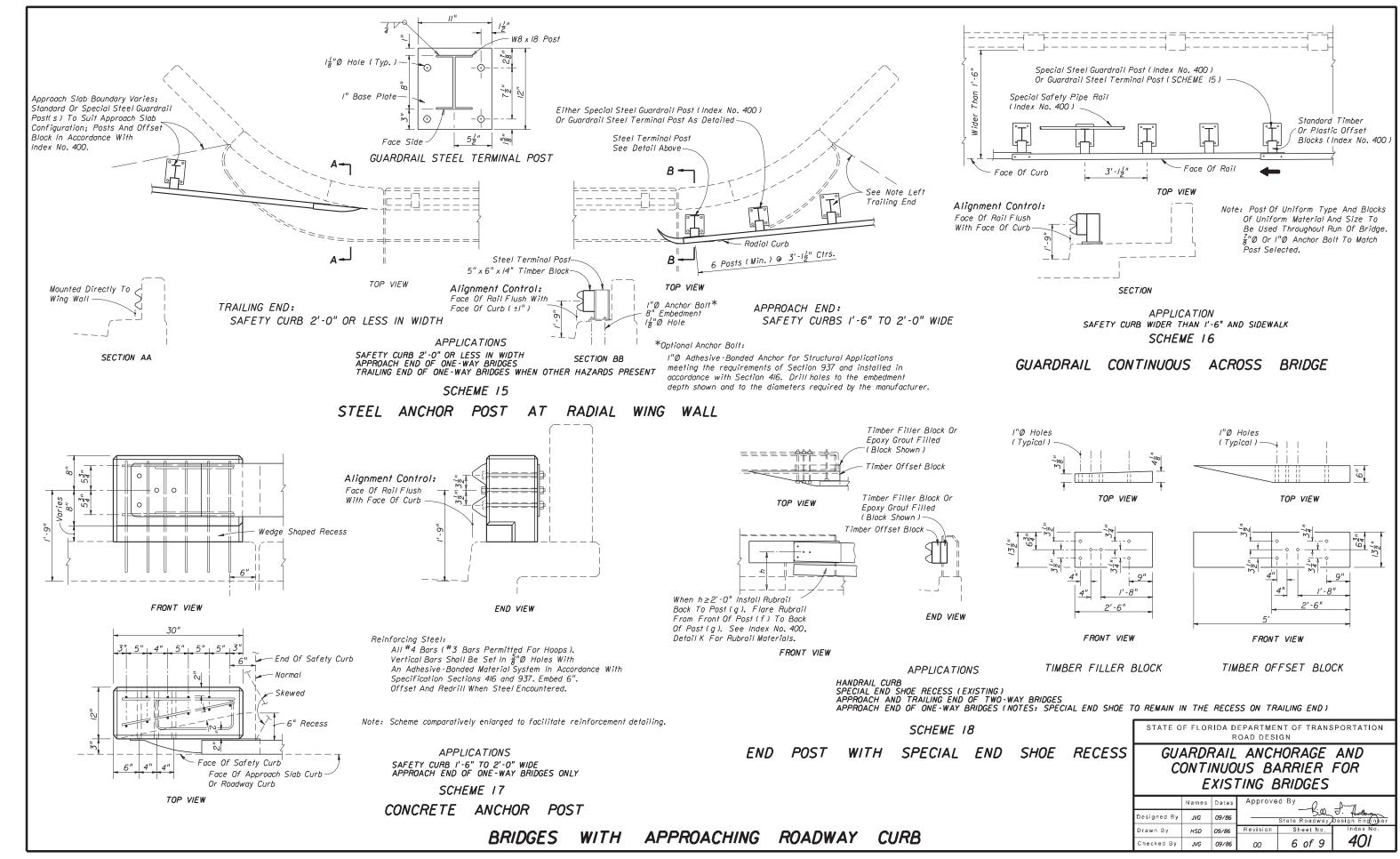
Shoulder Line

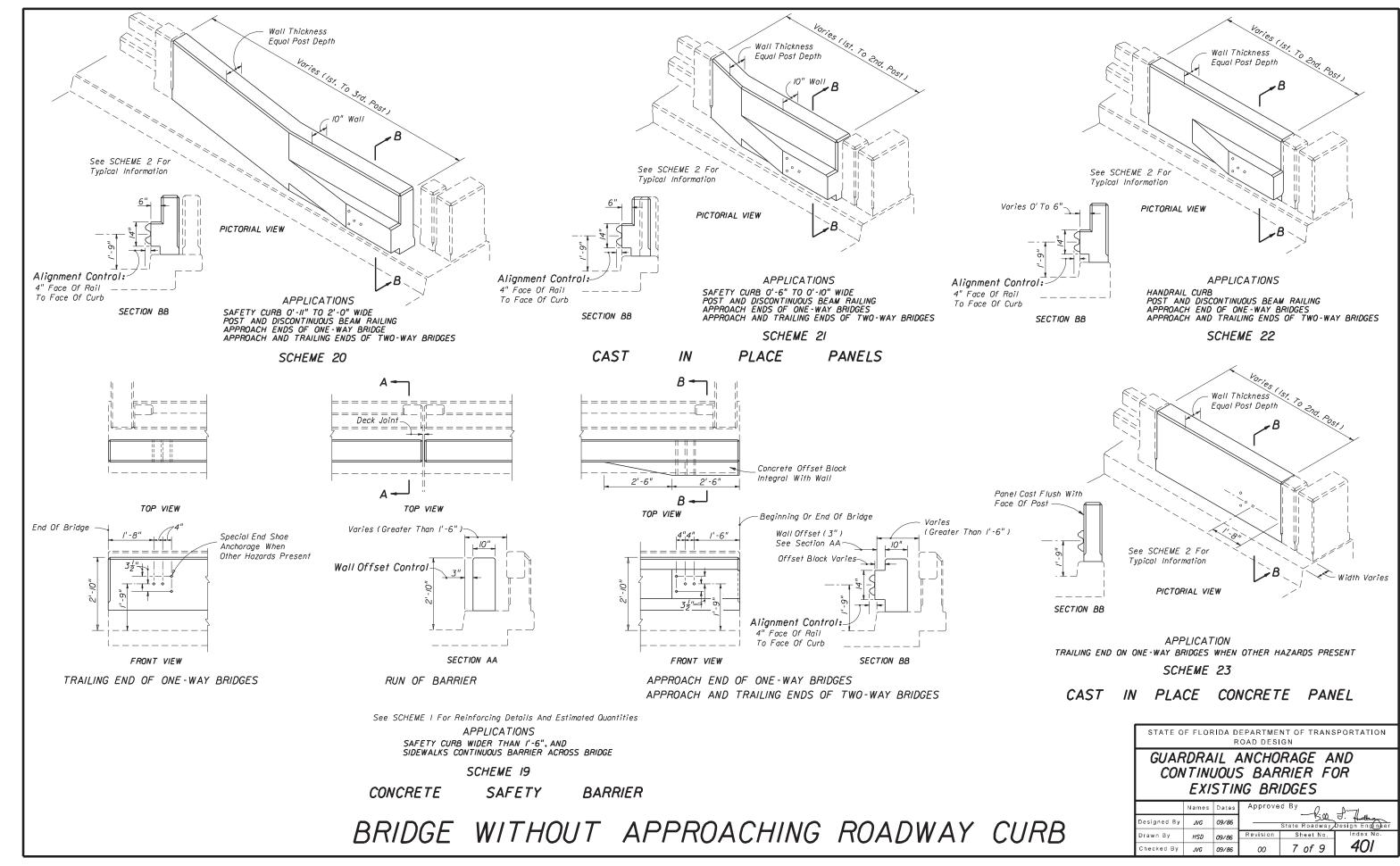
Same As SCHEME 13

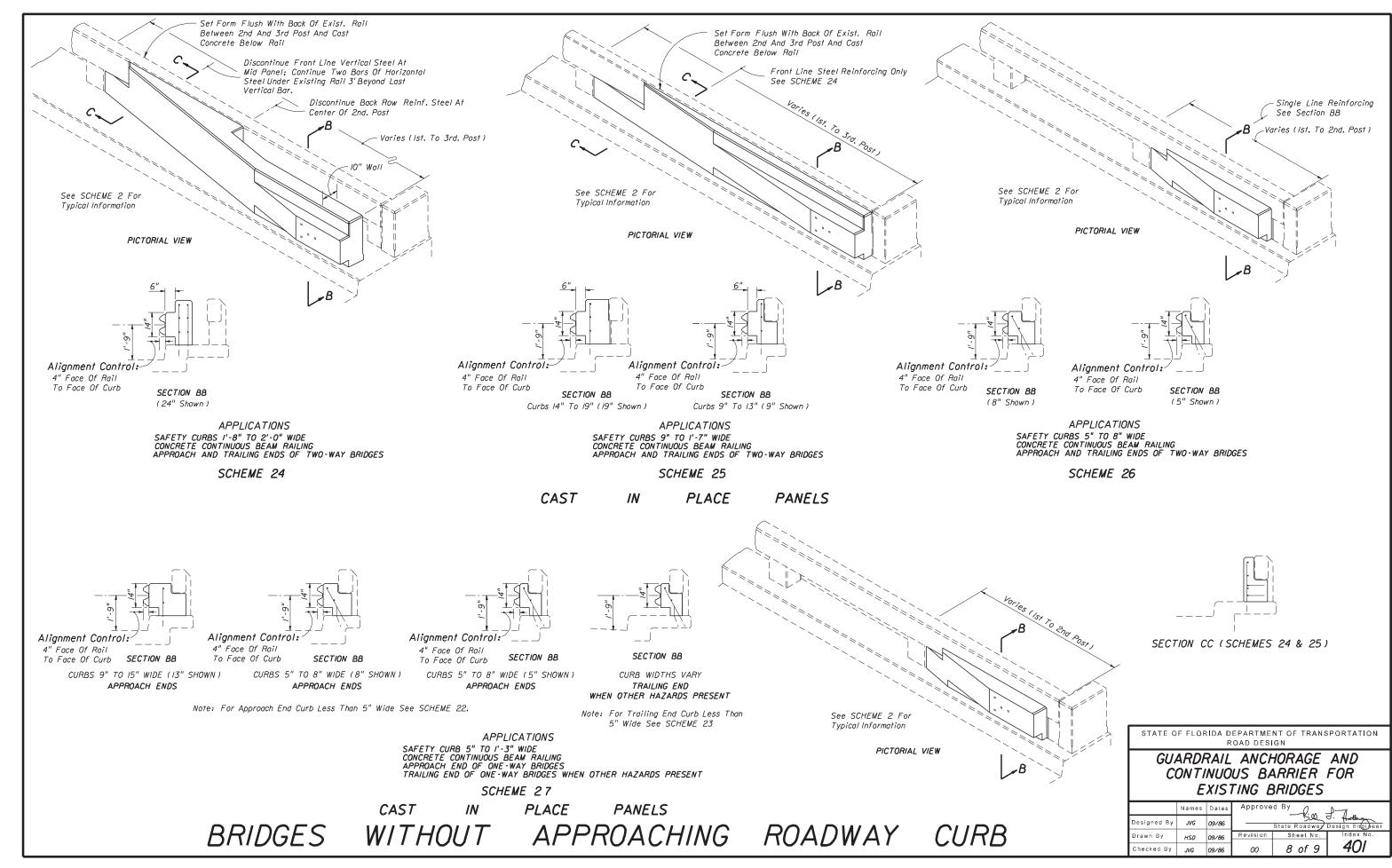
Edge Of Pavt.

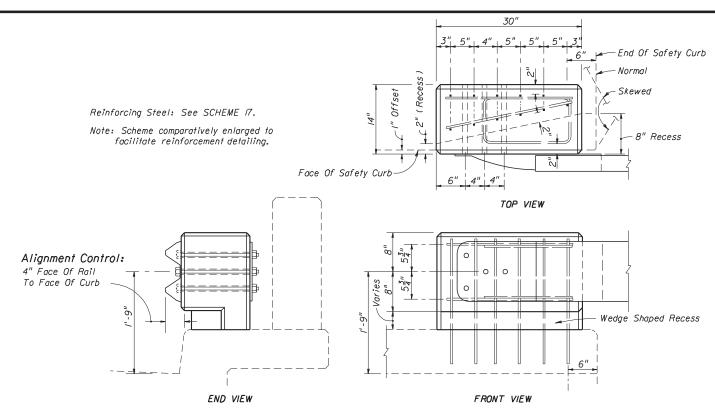
Closer Than 12" From Face Of Curb.

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Designed By	JVG	09/86		State Roadway	Design Engineer
Drawn By	HSD	09/86	Revision	Sheet No.	Index No.
Checked By	JVG	09/86	00	5 of 9	401



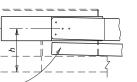






TOP VIEW

See SCHEME 18 For Complete Detailing



When $h \ge 2' - 0''$ Install Rubrail Back To Post (g). Flare Rubrail From Front Of Post (f) To Back Of Post (g). See Index 400, Detail K For Rubrail Materials.

FRONT VIEW

END VIEW

APPLICATIONS

HANDRAIL CURB

SPECIAL END SHOE RECESS (EXISTING)

APPROACH AND TRAILING END OF TWO-WAY BRIDGES

APPROACH END OF ONE-WAY BRIDGES (NOTE: SPECIAL END SHOE TO REMAIN
IN THE RECESS ON TRAILING END)

SCHEME 30

END POST WITH SPECIAL END SHOE RECESS

APPLICATIONS
SAFETY CURB WIDER THAN 1'-4" AND UP TO 2'-0"
APPROACH END OF ONE-WAY BRIDGES ONLY

SCHEME 28

CONCRETE ANCHOR POST

APPLICATIONS

SAFETY CURB WIDER THAN 9" AND UP TO 2'-0"
APPROACH AND TRAILING ENDS OF TWO-WAY BRIDGES
TRAILING END OF ONE-WAY BRIDGES WHEN OTHER HAZARDS PRESENT

SCHEME 29

See Index 410 For Holf Wall
Sectional Dimensions

2' Min. Shoulder Extension
Back Of Wall For Walls Set In
Soil Foundations Shall Be

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

GUARDRAIL ANCHORAGE AND CONTINUOUS BARRIER FOR EXISTING BRIDGES

ı		Names	Dates	Approve	. (/	1	
	Designed By	JVG	09/86		State Roadway	Design Engineer	
	Drawn By	HSD	HSD 09/86		Sheet No.	Index No.	
	Checked By	ecked By <i>JVG</i>		00	9 of 9	401	

Note (Scheme 29):

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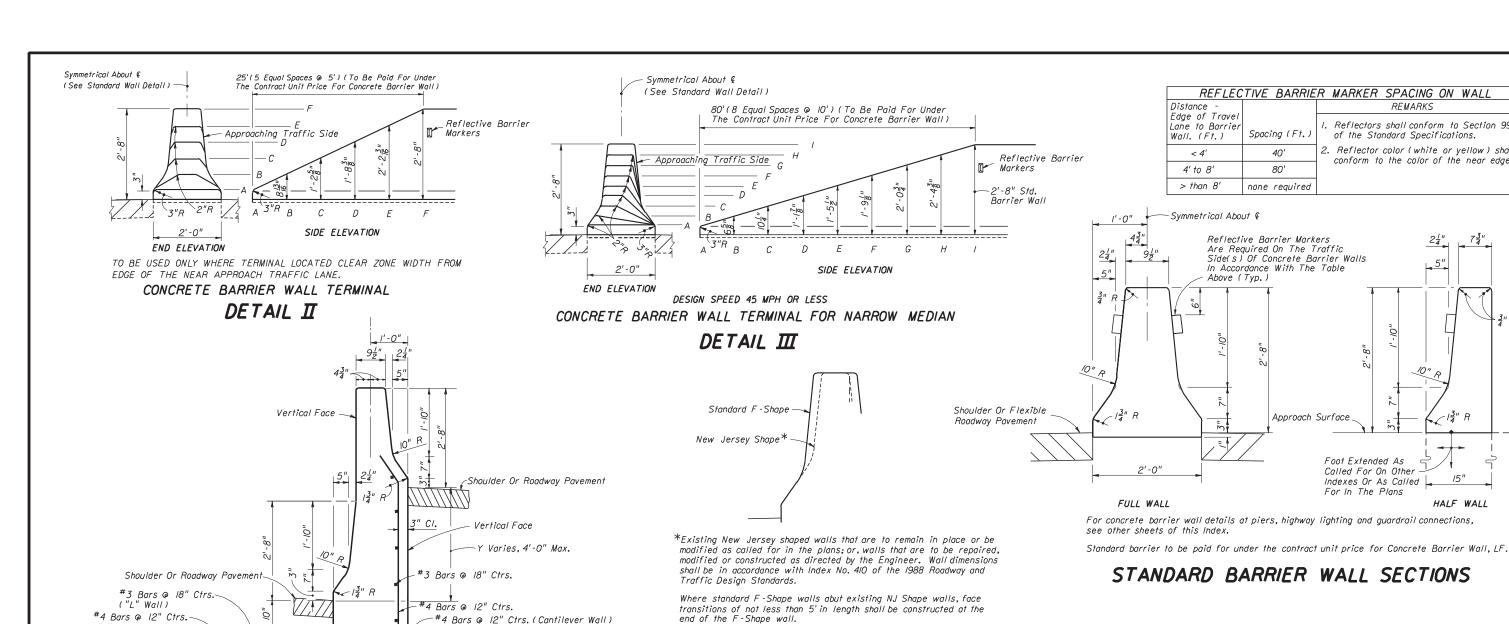
Portions of existing approach slab curbing, wingwalls, shoulder gutter, flumes and etc. may have to be revamped or removed.

Transition walls shall be reinforced in accordance with the 'Free End Reinforcement' detail of Index No. 410 with the hairpin bars spaced as shown in this pictorial. Walls mounted on existing approach slabs shall be anchored into the slabs with the front line of the hairpin reinforcement embedded 6" in the slab with an Adhesive-Bonded Material System in accordance with Specification Section 416 and 937. Offset and redrill when steel encountered.

Transition walls mounted on soil foundation shall have footings deepened 30" and the walls doweled into the end of the existing bridge in the following manner: Four I_4^{-1} diameter holes 6" deep shall be drilled in the end post of the existing bridge and #6 bars 15" long set in epoxy mortar. The holes shall be located as near as practical to the vertical center of the end of the transition wall and equally spaced to provide cover of 3" minimum. The ends of the dowels extending into the transition wall shall be wrapped with one layer of Type I asphalt-saturated roofing felt (15 $^{\#}$) with the ends crimped.

CONCRETE BRIDGE ANCHORAGE TRANSITION WALL

BRIDGES WITHOUT APPROACHING ROADWAY CURB



~#3 Bars @ 18" Ctrs.

'X Varies, 4'-10" Min., 5'-10" Max._

("L" Wall)

Design Criteria:

Concrete, LF.

constructed at the Contractor's ontion.

with 9" embedment in the footing.

Wall segments shall be

20' or more in length.

Const. Joint

X, Varies, 4'-0" Min., 6'-3" Max.

Vehicle: 4000 lbs., 60 mph, 25°, Avg. Lat. Impact Deceleration Force - 7G's (28 kips)

Vehicle Force Applications: 1000 lbs. Vert. At Top of Toe; 28 kips Horiz. At $5\frac{1}{2}$ " Above Pavt.

Unless the plans stipulate a specific wall type, either the cantilever wall or the "L" wall may be

Steel not required in walls of heights Y = 0' To 0' - 6'' when footing and stem cast as one unit. When footing and stem cast separately by construction joint, the footing joint surface shall be

Cost of the steel and concrete footing to be included in the contract unit price for Barrier Wall

Cantilever Wall Width X 4'-10" 5'-0" 5'-2" 5'-3" 5'-5" 5'-6" 5'-7" 5'-9" 5'-10"

MEDIAN BARRIER WALL FOR SUPERELEVATED SECTIONS

OR FOR VARIABLE ROADWAY PROFILE GRADES

Height Y 0'-0" 0'-6" 1'-0" 1'-6" 2'-0" 2'-6" 3'-0" 3'-6" 4'-0"

Width X₁ 4'-0" 4'-4" 4'-8" 5'-0" 5'-3" 5'-6" 5'-9" 6'-0" 6'-3"

roughened and #4 dowels 24" long installed at the centerline of the stem on 24" centers

WALL FACE SAFETY SHAPES

- Coating in accordance with Section 400 only when called for in the plans.
- d. Terminated in conjunction with a suitably designed transition to another barrier.
- 3. Expansion joints in wall required only at bridge ends and/or at locations where wall is an integral part of existing or proposed concrete slab; wall joints are to match an existing or proposed expansion joint.
- density as defined in the AASHTO T-99 specifications.
- 5. Cast-in-place barrier wall normally will be a continuous pour without transverse contraction joints. Cast-in-place segments with
- 6. Precast construction is allowed as an alternate to cast-in-place construction.
 - a. Wall segments < 40' in length shall be joined by a transverse joint in accordance with Details C & D. The minimum segment length is 20'.
 - c. Reinforcement may be required for handling stresses.
- See individual details for pay item information.
- 8. For barrier wall inlet details see Indexes Nos. 217, 218 and 219.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

REFLECTIVE BARRIER MARKER SPACING ON WALL

Spacing (Ft.)

40'

80'

none required

Reflective Barrier Markers

Are Required On The Traffic Side(s) Of Concrete Barrier Walls In Accordance With The Table

Approach Surface.

STANDARD BARRIER WALL SECTIONS

Foot Extended As Called For On Other

Indexes Or As Called For In The Plans

Reflectors shall conform to Section 993-5

conform to the color of the near edgeline

24"

2. Reflector color (white or yellow) shall

of the Standard Specifications.

Distance -

Edge of Travel

Lane to Barrier

Wall. (Ft.)

< 4'

4' to 8'

> than 8'

43"

92"

FULL WALL

-Symmetrical About €

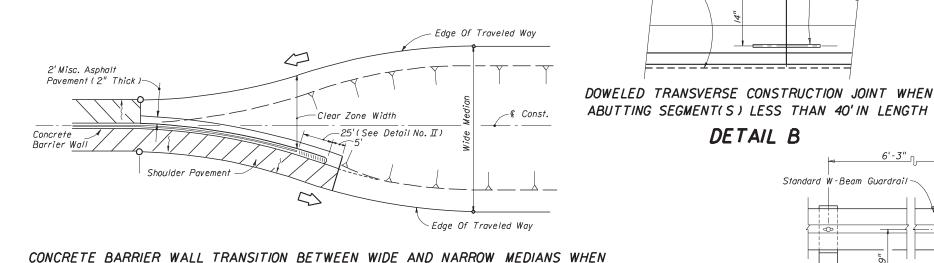
Above (Typ.)

CONCRETE BARRIER WALL

	Names	Dates	Approve	a By	_1/			
Designed By			State Roadway Design Engineer					
Drawn By	awn By AF/HSD		Revision	Sheet No.	Index No.			
Checked By	LMF/JG	73/91	00	1 of 22	410			

GENERAL NOTES

- I. Class II concrete shall be used for all reinforced and plain (nonreinforced) concrete barrier walls; except, in moderately and extremely aggressive environments, Class IV concrete shall be used. Exposed concrete surfaces shall have a Class 3 surface finish in accordance Section 52I of the Standard Specifications, unless other finish called for in the plans. The surfaces shall have a Class 5 Applied Finish
- Concrete barrier wall terminal notes for design speeds ≥50 mph.
 a. Terminated outside clear zone of the appproach traffic with 'DETAIL II' end treatment.
 - b. Terminated within a shielded location.
 - c. Terminal protection by the use of a crash cushion system.
- 4. When the barrier is installed adjacent to the pavement the top I2" of the subgrade shall be compacted to at least IOO% of the
- a length <40' shall be joined to adjacent sections by doweling. See Detail B.
- b. Bedding of the precast sections shall be facilitated by the use of sand-cement grout or equal method to assure uniform bearing.
- 7. Cost of reinforcing steel and reflective barrier markers shall be included in the contract unit price for concrete barrier wall.



BARRIER WALL END LOCATED OUTSIDE APPROACH CLEAR ZONE OR HORIZONTAL CLEARANCE

Misc. Asphalt Pavt.

Extended Shoulder

Crash Cushion (Type Varies, See Plans

(Full, Half Or Trapezoidal Wall)

Concrete Barrier Wall-

Not Steeper Than I: 10

-Edge Of Traveled Way

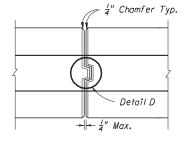
-Normal Shoulder Line

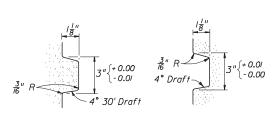
Extended Shoulder Break Point

15° Or Flatter

 \bigcirc

Cold Joint Plain Steel Dowels I"Ø At € Of Wall; Lubricate As Per Specifications





TOP VIEW

PRECAST BARRIER TRANSVERSE JOINTS

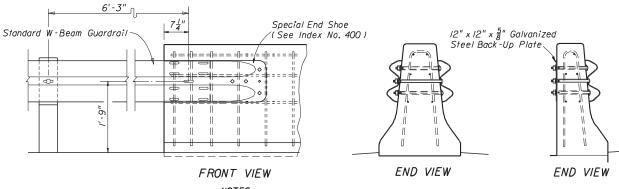
TOP VIEW

STRAIGHT TONGUE AND GROOVE

DETAIL C

DETAIL D

DETAIL B



NOTES

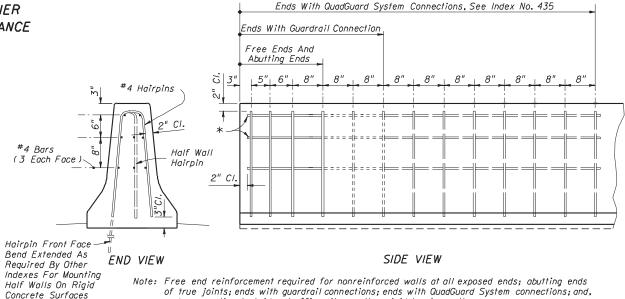
- I. End of wall flush mounted connections are not applicable to two-lane two-way facilities. See Sheets I8 and 20 for trailing end connections on two-lane two-way facilities and for approach guardrail connections.
- 2. Trailing guardrail connections to double face safety shaped walls will be under one of the following traffic conditions and mounting methods:
 - (a) One-way traffic trailing condition one side only flush mount with flat steel back-up plate on back side. (b) One-way traffic trailing condition both sides flush mount both sides.

 - (c) For trailing condition one side and approach traffic condition opposite side see "Median Barrier Wall" mounting, Sheet 20.

SHOULDER TREATMENT WHEN CRASH CUSHIONS SHIELDING CONCRETE BARRIER WALL END LOCATED INSIDE APPROACH CLEAR ZONE OR HORIZONTAL CLEARANCE

DETAIL A

W-BEAM GUARDRAIL CONNECTION TO CONCRETE BARRIER WALL TRAILING ENDS

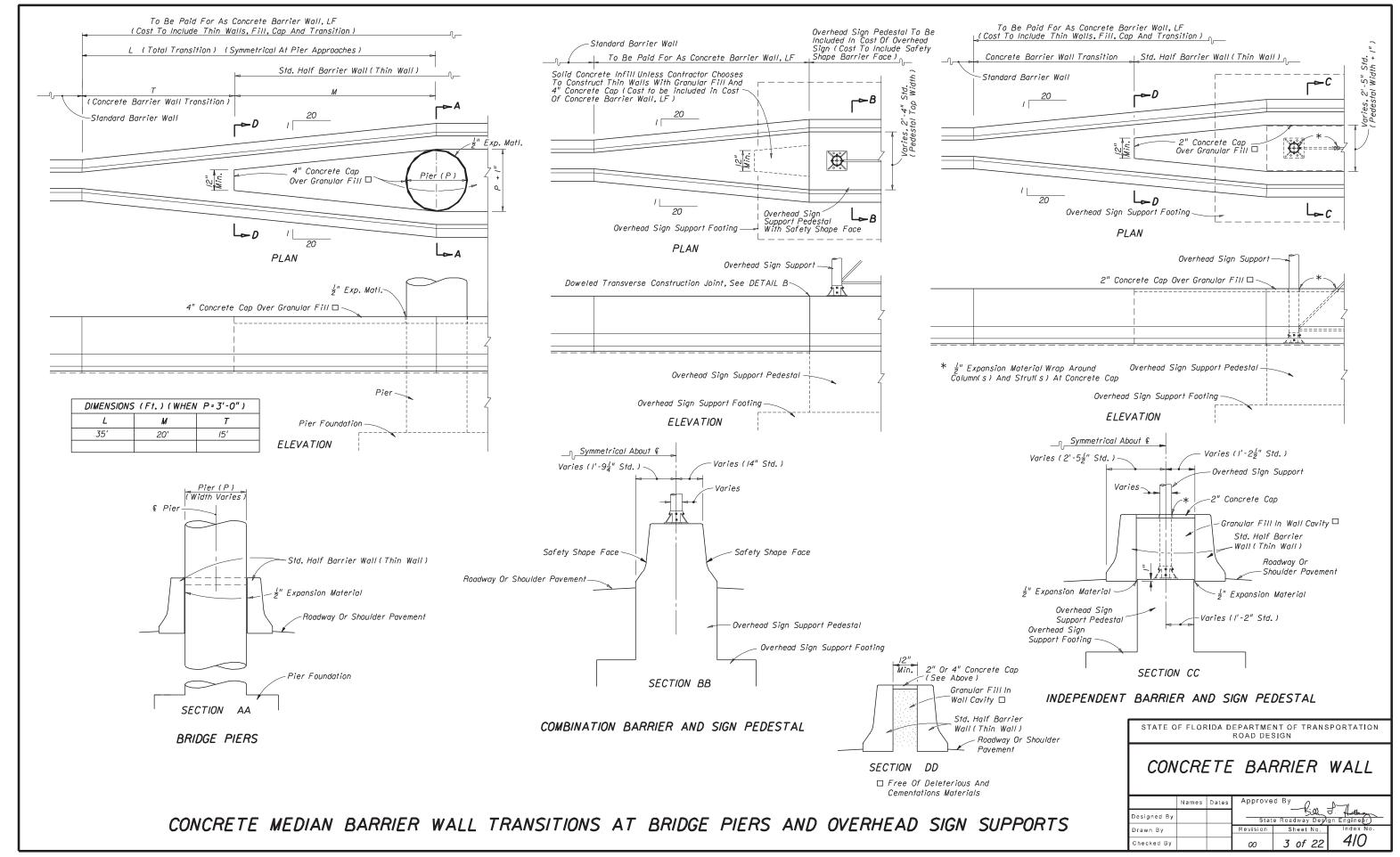


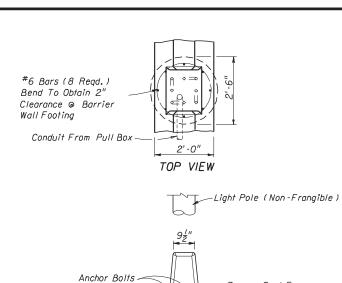
ends connecting to bridge traffic rails or other rigid barrier walls.

FREE END REINFORCEMENT

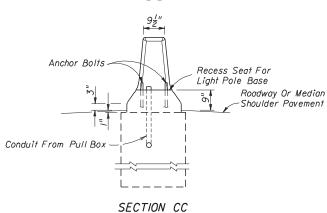
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

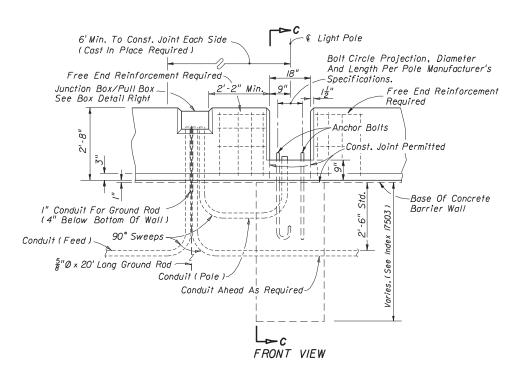
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Designed By				State Roadway	Design Ergineer
Drawn By	AF/HSD	73/9/	Revision	Sheet No.	Index No.
Checked By	LMF/JVG	73/91	00	2 of 22	410





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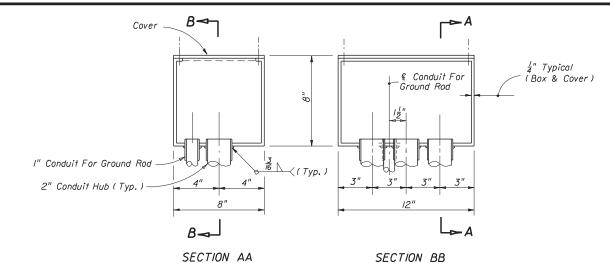


Note: For foundation design and details see Index No. 17503.

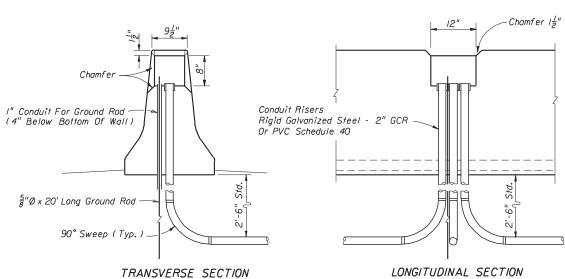
Refer to Highway Lighting Plans for size of conduit.

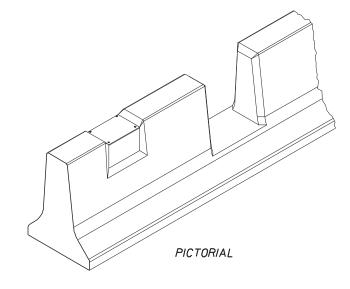
Payment for the 2'-6" concrete shaft including reinforcing steel, anchor bolts and accessories shall be included in the contract unit price for Light Pole Complete, EA.

LIGHT POLE MOUNTING IN MEDIAN BARRIER WALL



JUNCTION BOX





INSTALLATION

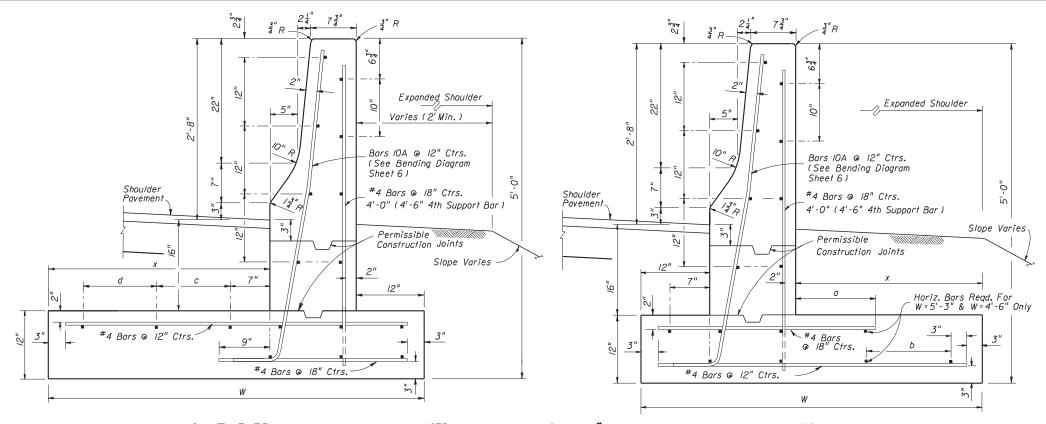
JUNCTION BOX NOTES

- I. Junction boxes are to be fabricated from steel conforming to ASTM A36 and be hot dipped galvonized after fabrication. All seams shall be continuously welded and ground smooth. A neoprene gasket shall be attached to the box to provide a watertight cover. The cover screws shall be fully galvanized.
- 2. Remove excess concrete while green and hand form chamfers.
- 3. Junction box complete and conduit risers are incidental to the construction and cost of the barrier wall; there is to be no separate compensation for the box, risers or installation unless specifically called for in the plans.

JUNCTION BOX - ELECTRICAL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

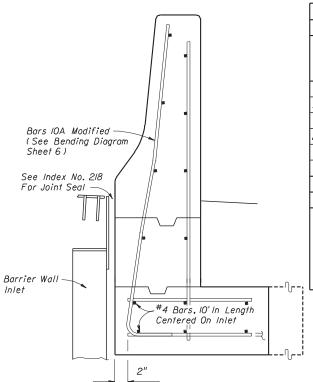
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Designe	Ву				State Roadway	Design Engineer
Drawn B	у	HSD	9/85	Revision	Sheet No.	Index No.
Checked	Ву	JVG	9/85	00	4 of 22	410



CANTILEVER WALL

NOTE: All longitudinal reinforcement #4 bars.

L-WALL



REINFORCING STEEL MODIFICATIONS AT BARRIER WALL INLETS (INDEX NO. 218)

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							DIMENSIONS AND	J QUANT	IIIES						
	CANTILEVER WALL								L-WALL						
Lengi Of Barri Wal	er w		х	С	d	Class II Concrete CY Per Lin. Ft.	Reinforcing Steel LBS. Per Lin. Ft.	Length* Of Barrier Wall	W	х	а	Ь	Class II Concrete CY Per Lin. Ft.	Reinforcing Ste LBS. Per Lin. F	
≥ 40'	3'-3	"	1'-0"	NA	NA	0.27	18	≥ 40'	3'-3"	1'-0"	6"	NA	0.27	18	
35' to	39' 3'-6	" /	1'-3"	NA	NA	0.28	18	35' to 39'	3'-6"	1'-3"	6"	NA	0.28	18	
30' to	34' 4'-0	" /	1'-9"	NA	NA	0.29	19	30' to 34'	3'-9"	/'-6"	6"	NA	0.29	18	
25' to	29' 4'-6	" 2	2'-3"	/4"	NA	0.3/	20	25' to 29'	4'-0"	1'-9"	9"	NA	0.30	19	
21' to	24' 5'-0	" 2	2'-9"	18"	NA	0.33	20	20' to 24'	4'-6"	2'-3"	12"	12"	0.31	20	
19' &	20' 5'-6	" :	3'-3"	13"	/3"	0.35	21	15' to 19'	5'-3"	3'-0"	16"	17"	0.34	21	
17' &	18' 6'-0	" [3'-9"	16"	16"	0.37	21								
15' &	16' 6'-6	" 4	4'-3"	18"	18"	0.39	22								
	•		0	~+:+:	.c. ch	own are for informa	tion only. For mothe	d of naumo	n+ coo	0011000	4 004	o bol	O		

DIMENSIONS AND OUANTITIES

Quantities shown are for information only. For method of payment see payment note below.

Barrier wall inlets (Index 218) shall be isolated from the barrier wall stem and footing by I" expansion material.

*Any length less than 40' must be a continuous (non-jointed) segment. Walls of 40' or more in length may be made up of segments of 20' or more in length provided the segments are joined by a transverse joint in accordance with Detail B. Sheet 2; segments shall have dimensions same as wall \geq 40' above.

PAYMENT:

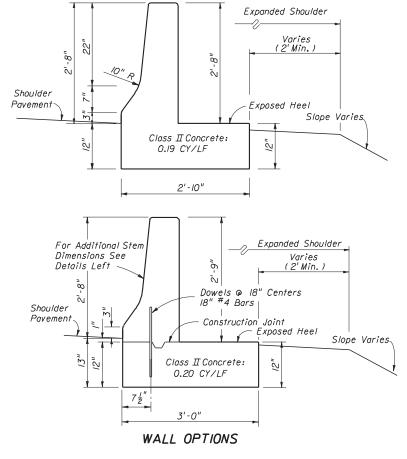
Wall to be paid for under the contract unit price for Concrete Barrier Wall (Rigid-Shoulder), LF.

DESIGN NOTES:

Use of this barrier wall should be limited to special applications such as hazard encroachment into the clear zone where barrier wall deflection, rotation or translation cannot be tolerated; example hazards to consider are as follows:

(a) Structure supporting piers, bents and pylons (b) Pumping, metering, control or other similar critical stations (c) Quarries (d) Intolerable vertical drops (e) Historic structures or monuments (f) Rail transit travel way or passenger station (g) Other similar occupancies

REINFORCED CONCRETE BARRIER WALL (SHOULDER)



NOTE:

Wall to be paid for under the contract unit price for Concrete Barrier Wall (Plain-Shoulder), LF.

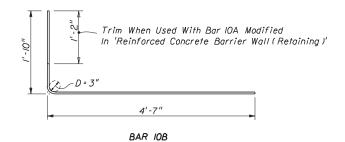
DESIGN NOTE

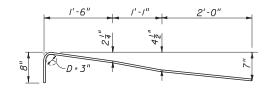
Wall shall have a length of 40' or greater. Wall of 40' or more in length may be made up of segments of 20' or more in length provided the segments are joined by a transverse joint in accordance with Detail B, Sheet 2; seaments shall have dimensions same as wall shown above.

PLAIN CONCRETE BARRIER WALL (SHOULDER)

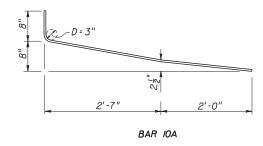
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By	P-71
Designed By				State Roadway/	Design Engineer
Drawn By	HSD	9/85	Revision	Sheet No.	Index No.
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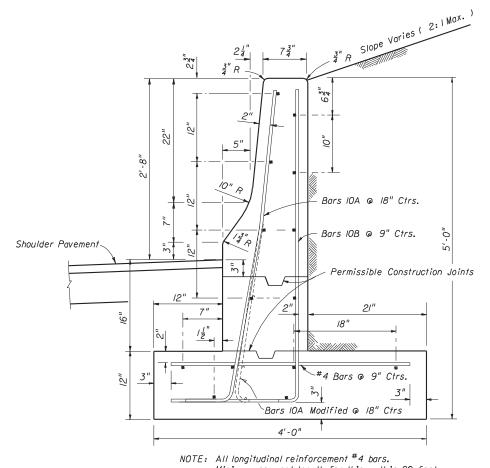




For Use In Areas Where Obstructions Require Localized Omission Of Toe BAR IOA MODIFIED



BENDING DIAGRAMS



OTE: All longitudinal reinforcement #4 bars.

Minimum segment length for this wall is 20 feet.

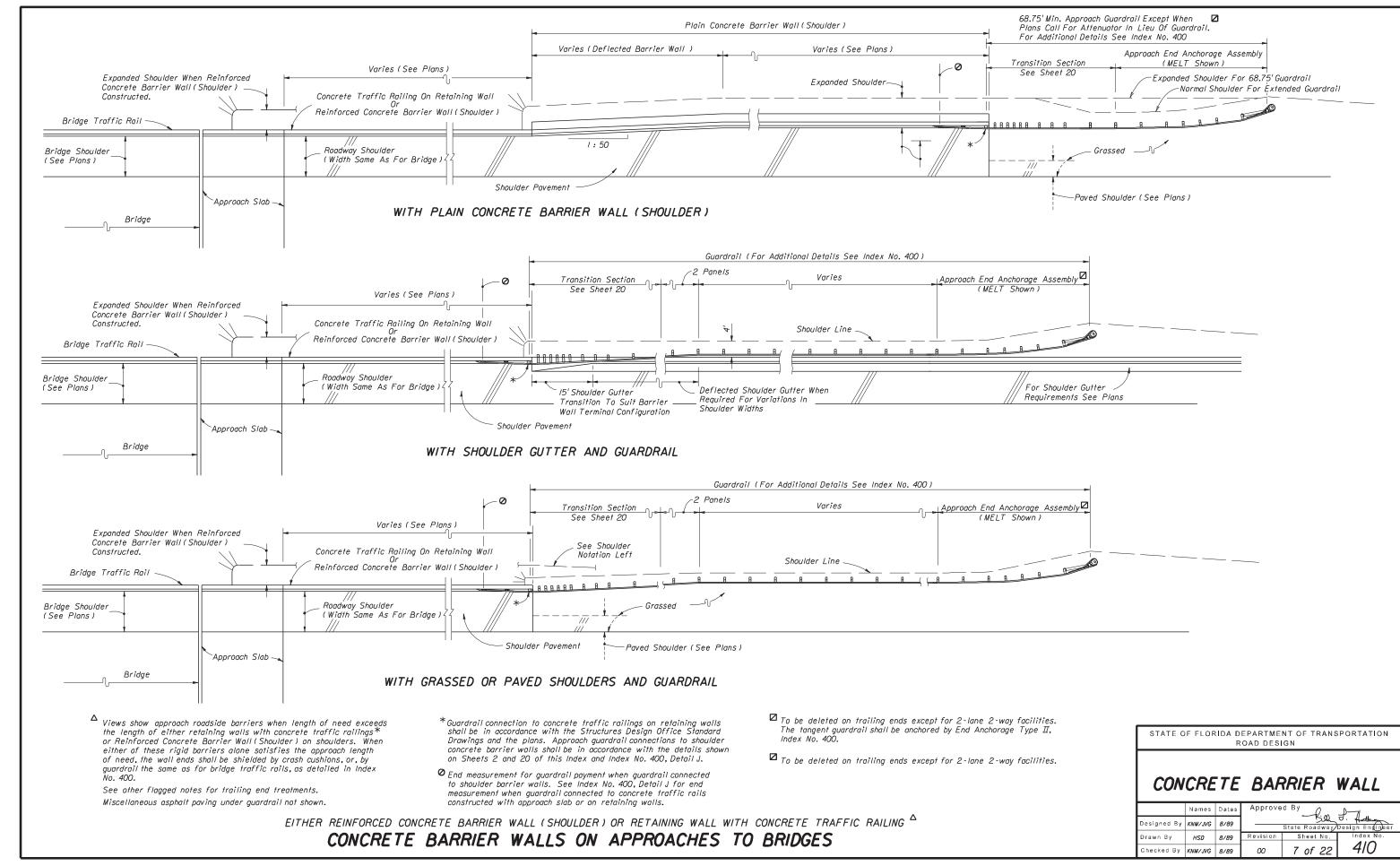
Wall to be paid for under the contract unit price
for Concrete Barrier Wall (Rigid-Retaining), LF.

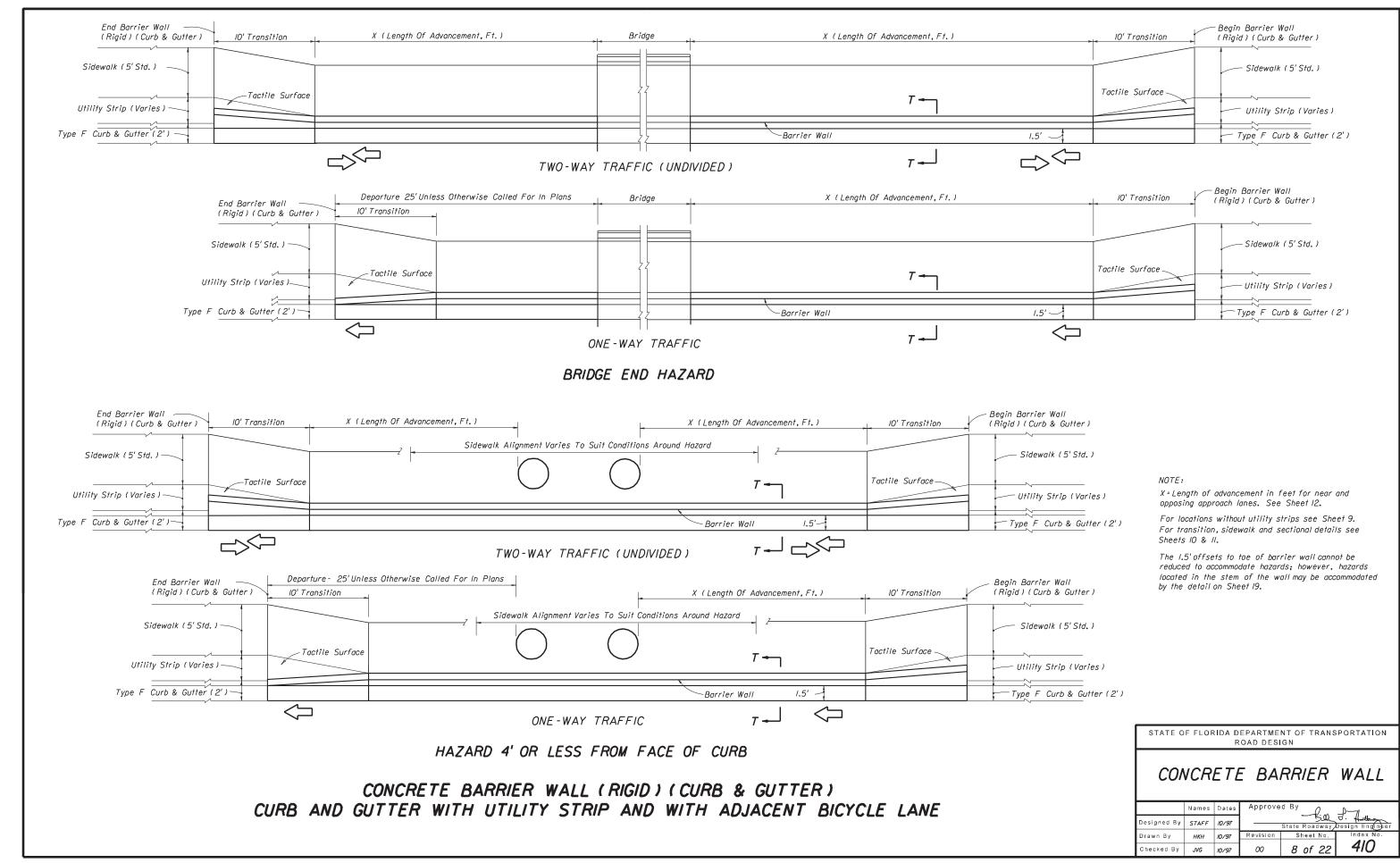
QUANTITIES: Class II Concrete 0.29 CY/LF Reinforcing Steel 21 LBS/LF

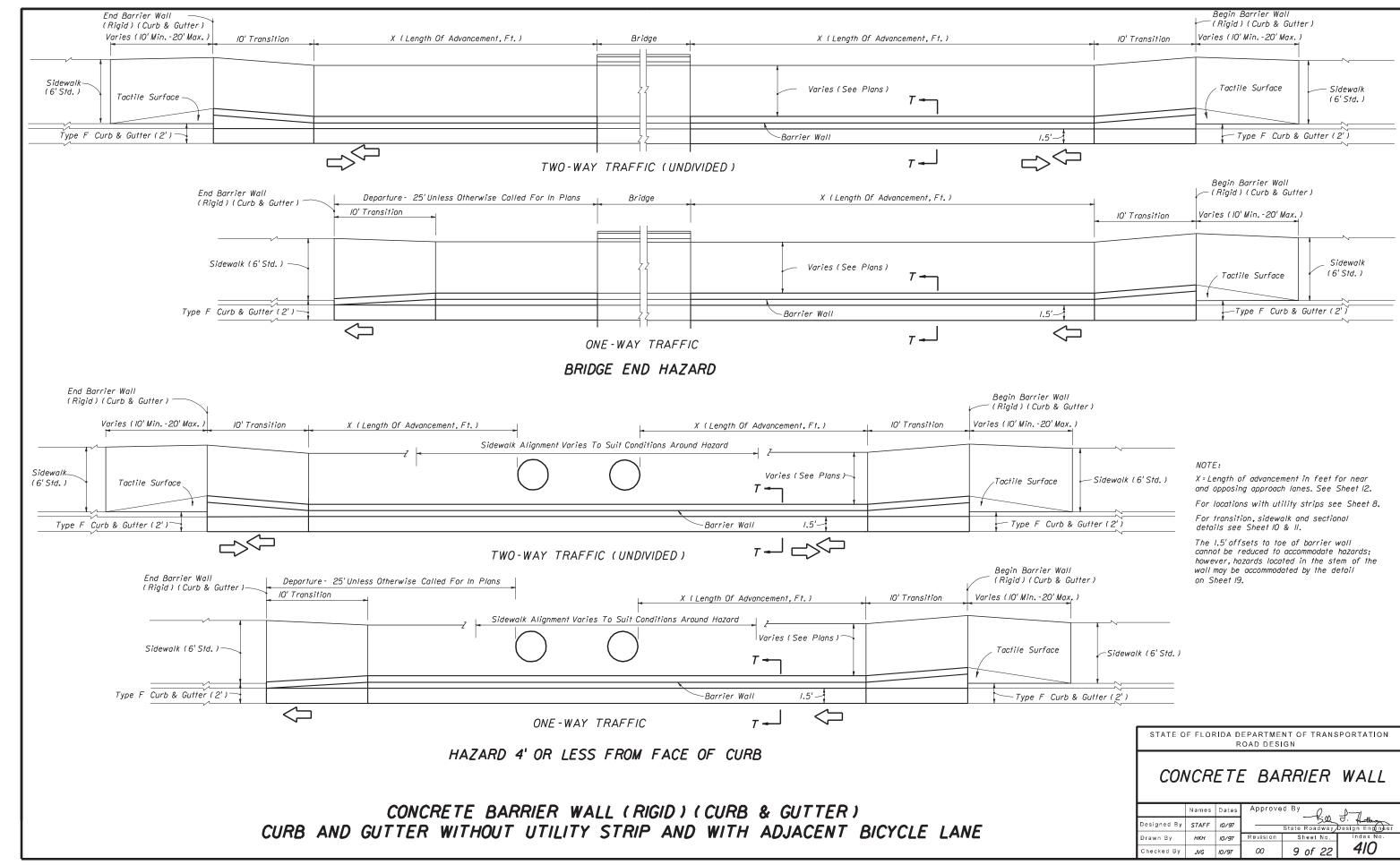
REINFORCED CONCRETE BARRIER WALL (RETAINING)

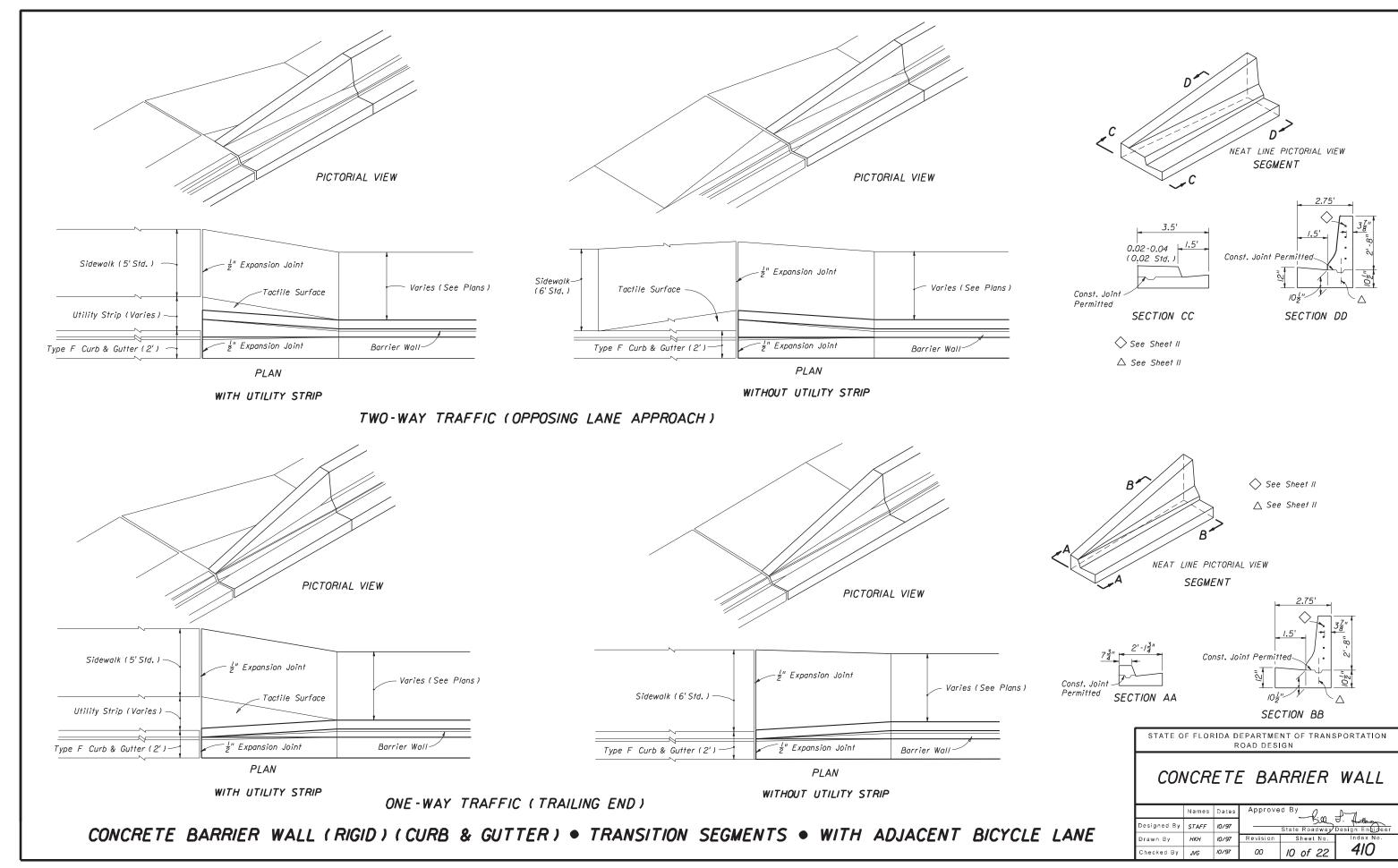
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

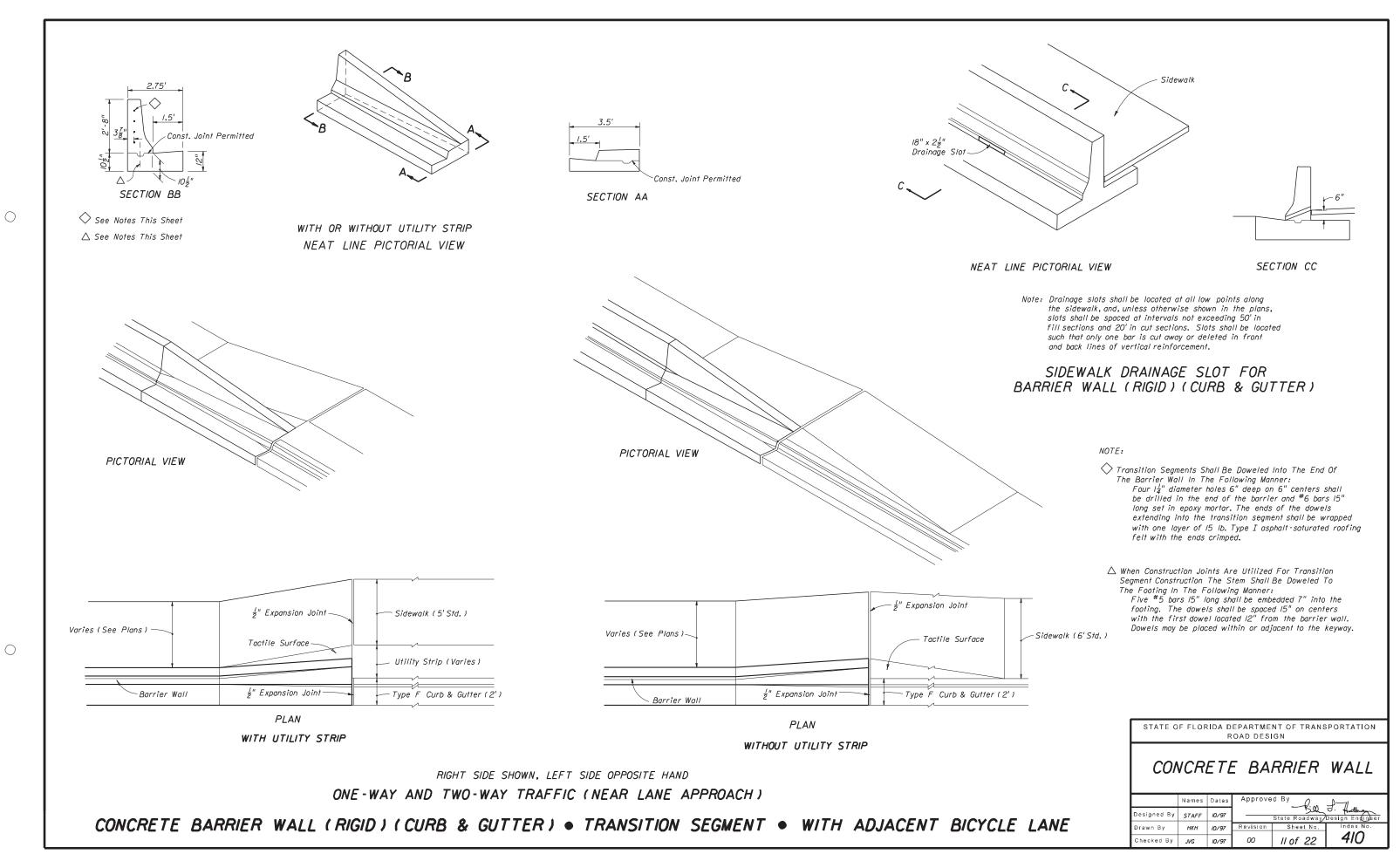
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Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	6 of 22	410

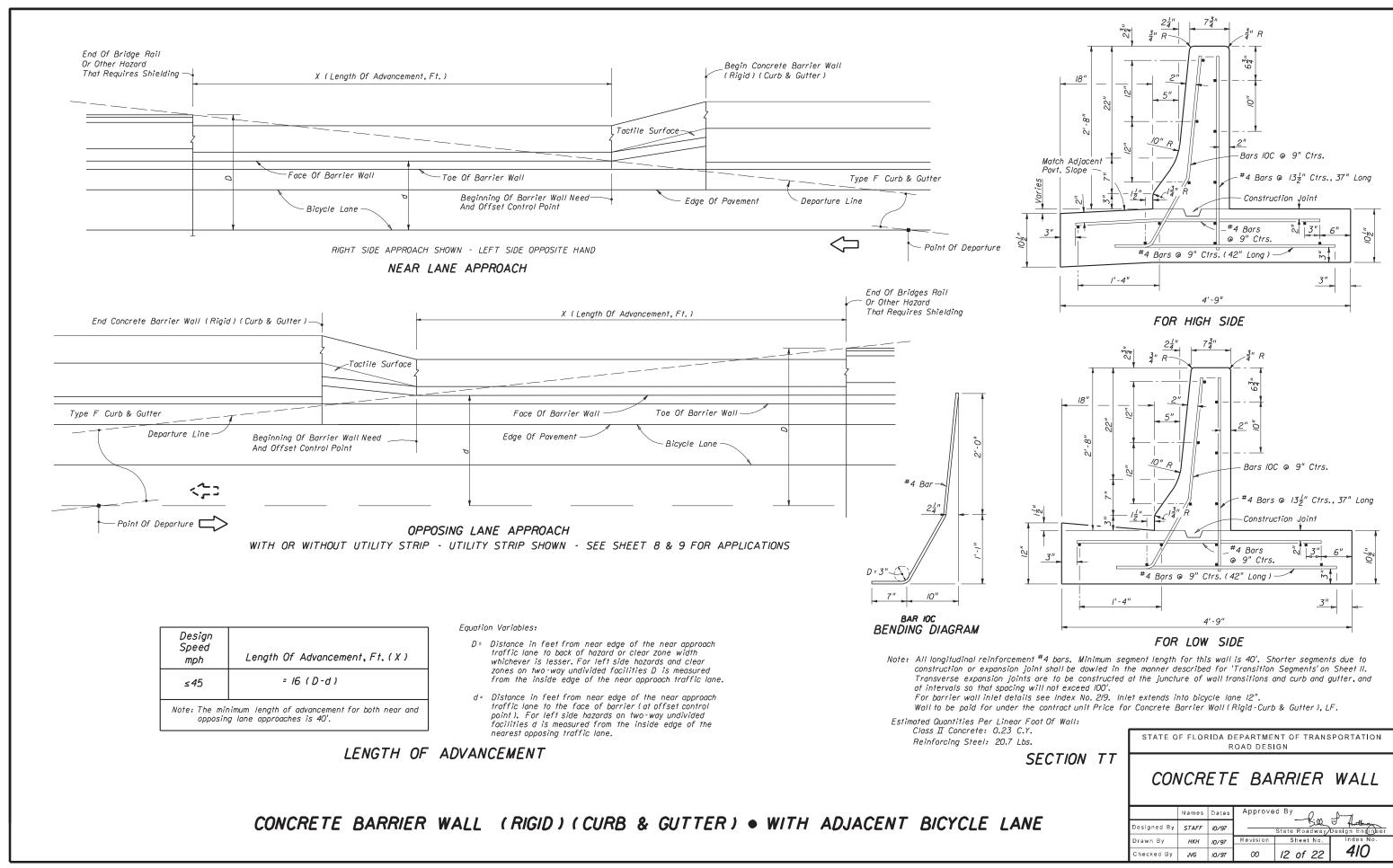


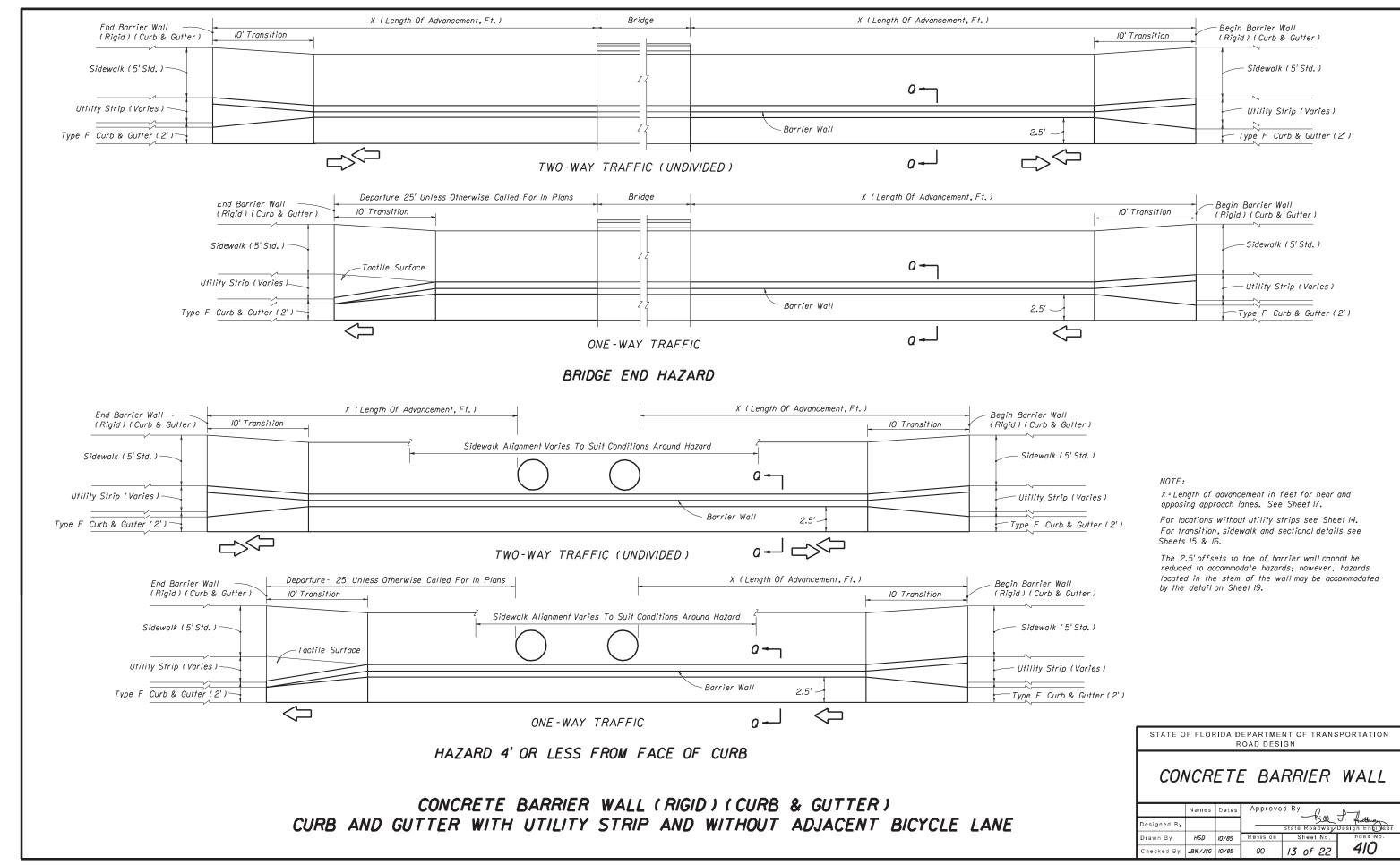


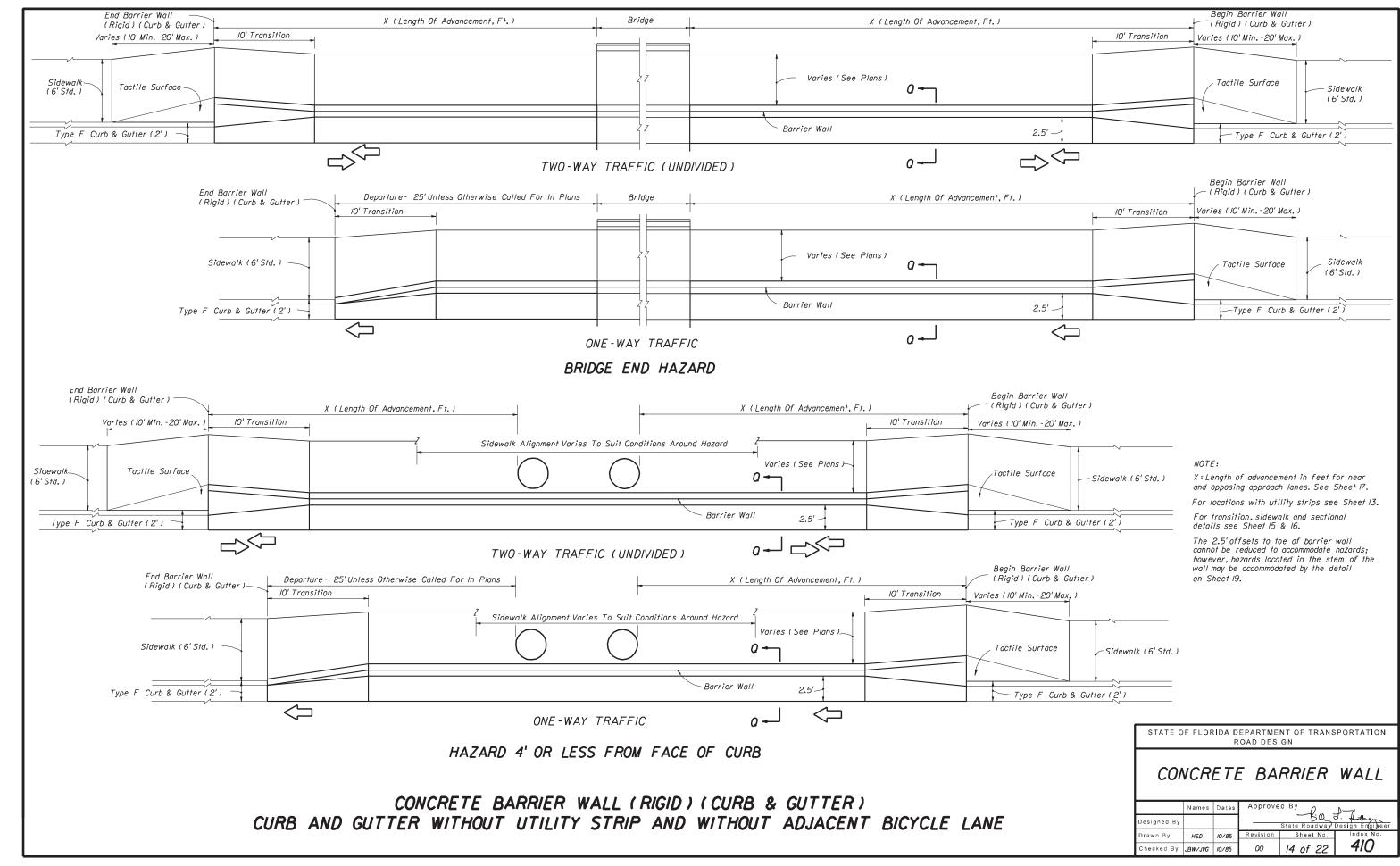


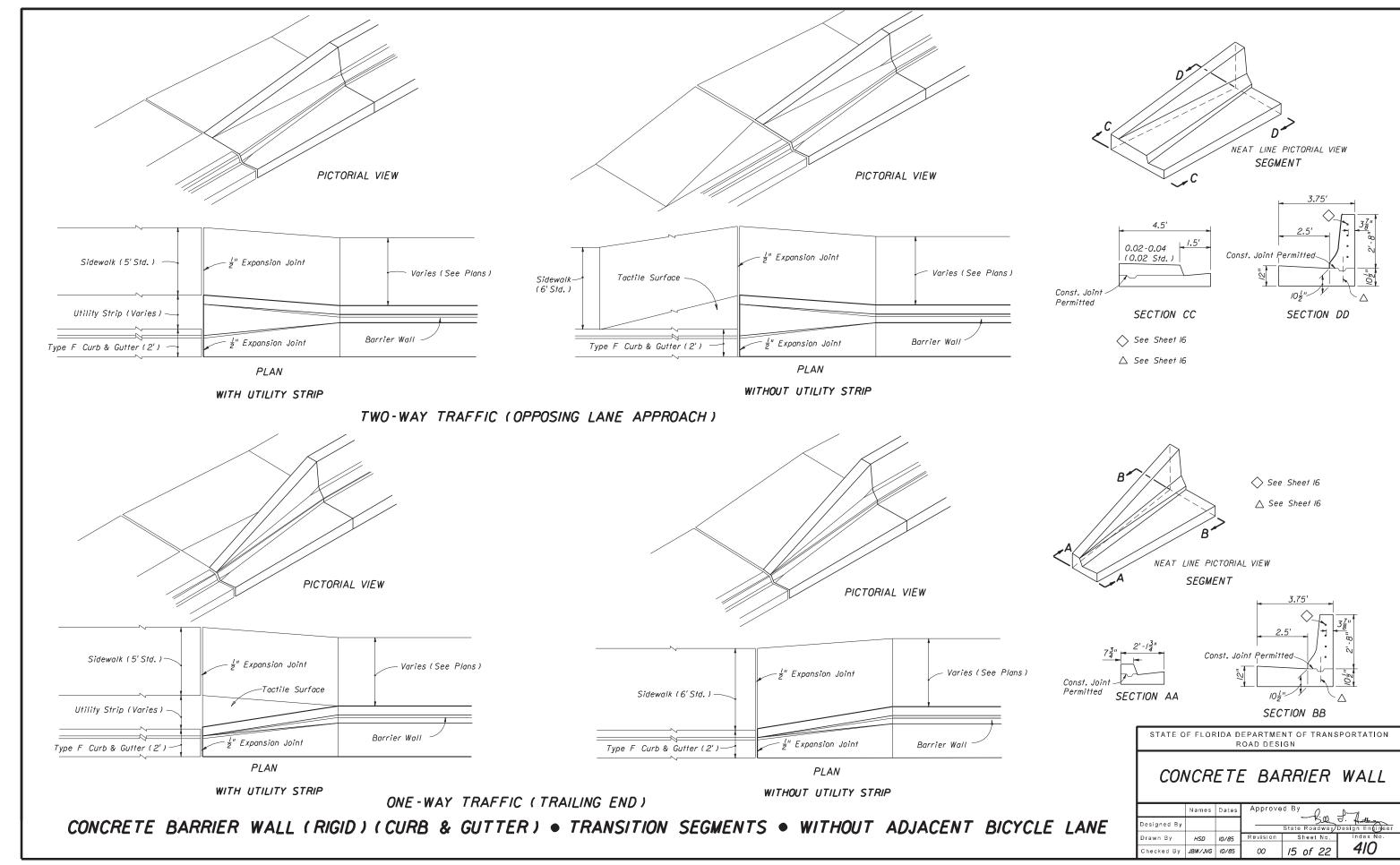


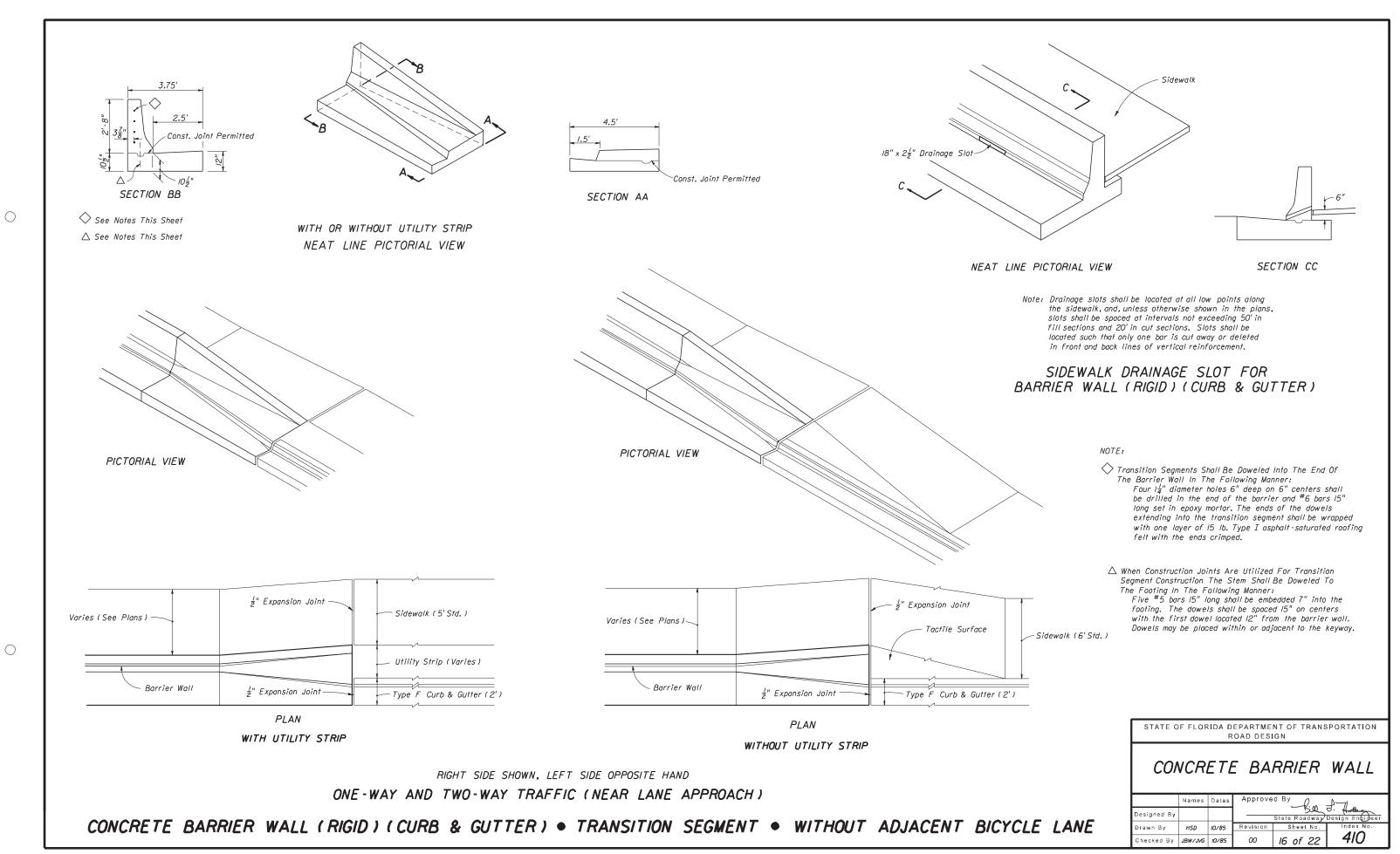


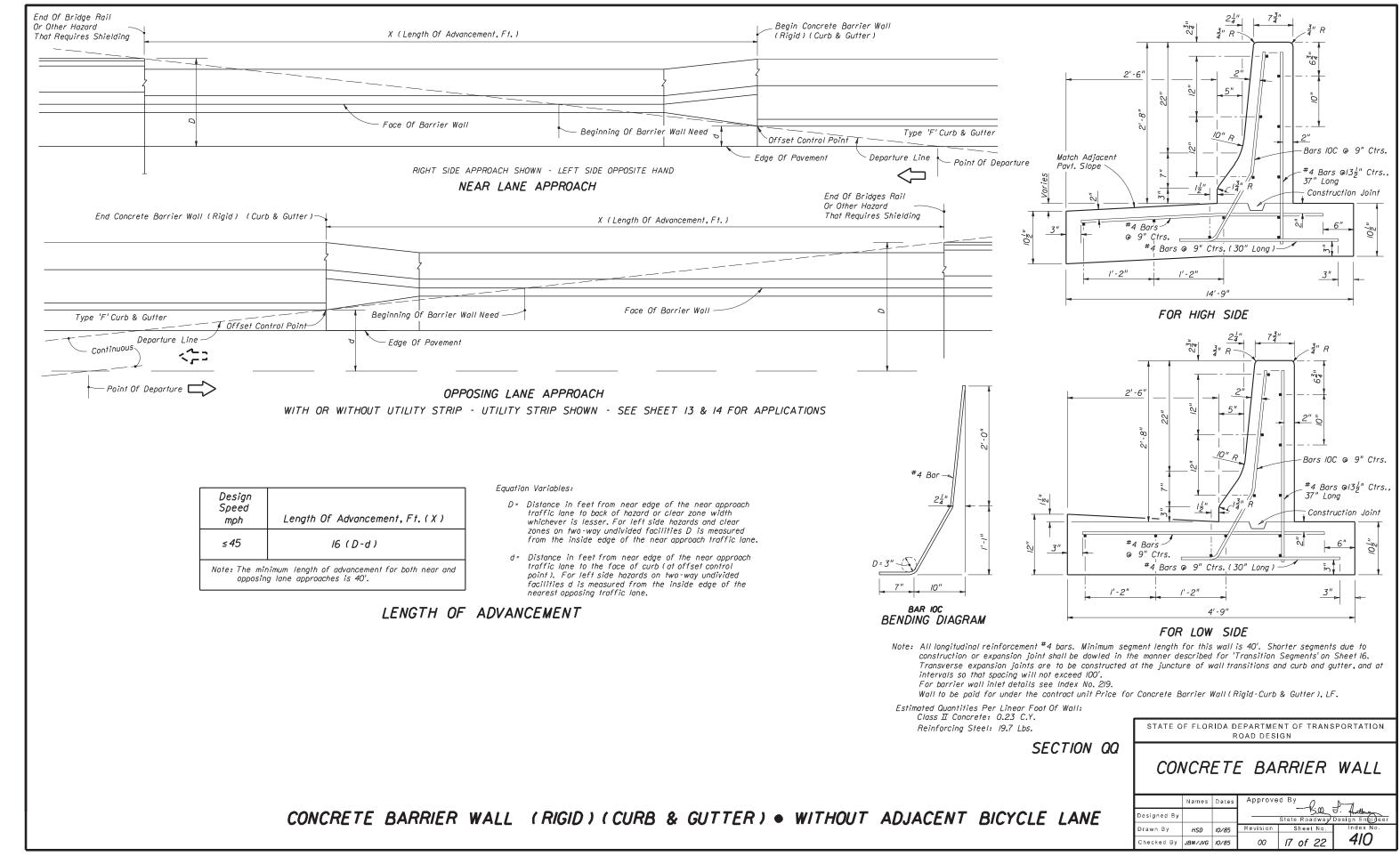


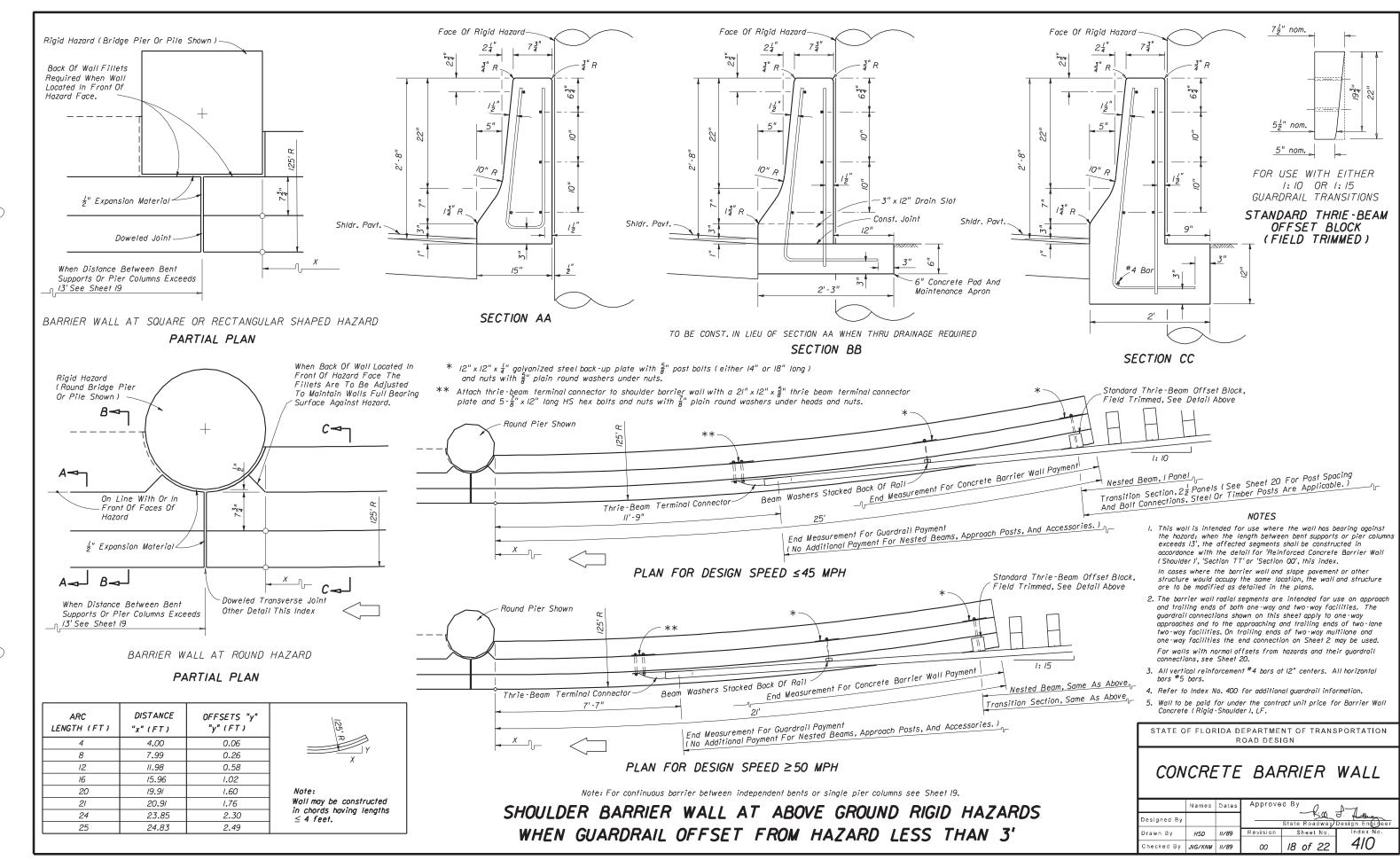


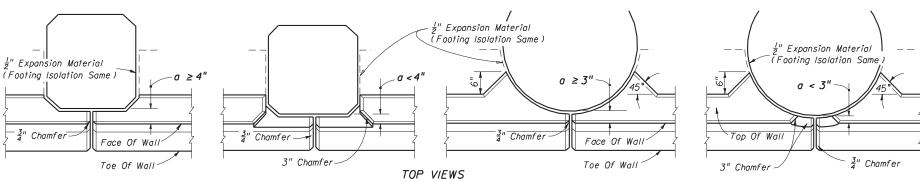






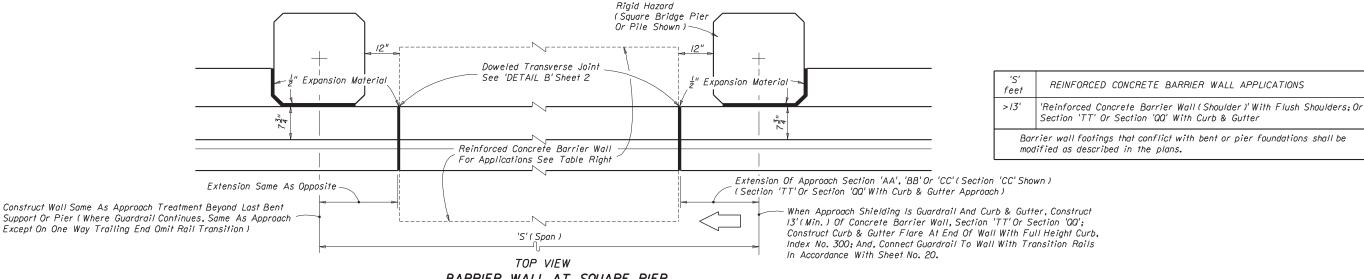




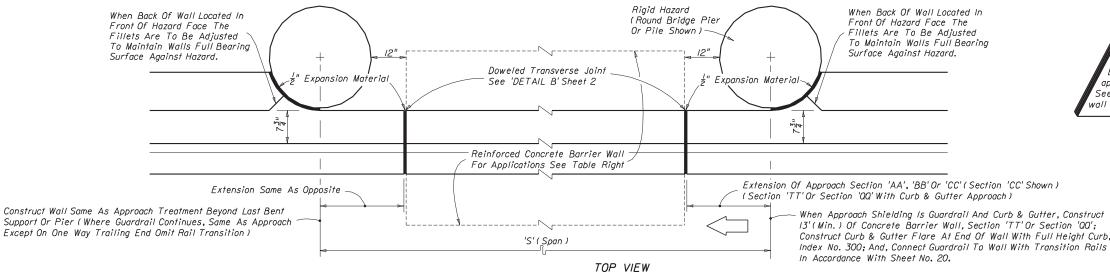


'a' Varies (Circular Or Octagonal Hazard Not More Than 2" In Front Of Face Of Wall) Applicable To Sections 'AA' And 'BB' With Spans Of ≤13', And To Section 'CC', Sheet No. 18. Applicable To Other Rigid Walls Of This Index For Spans >13' Unless Otherwise Shown In The Plans.

HAZARD PENETRATING STEM OF RIGID CONCRETE BARRIER WALLS



BARRIER WALL AT SQUARE PIER



BARRIER WALL AT ROUND PIER

CONCRETE BARRIER WALL WHEN SPAN BETWEEN BENT SUPPORTS OR PIER COLUMNS EXCEEDS 13'

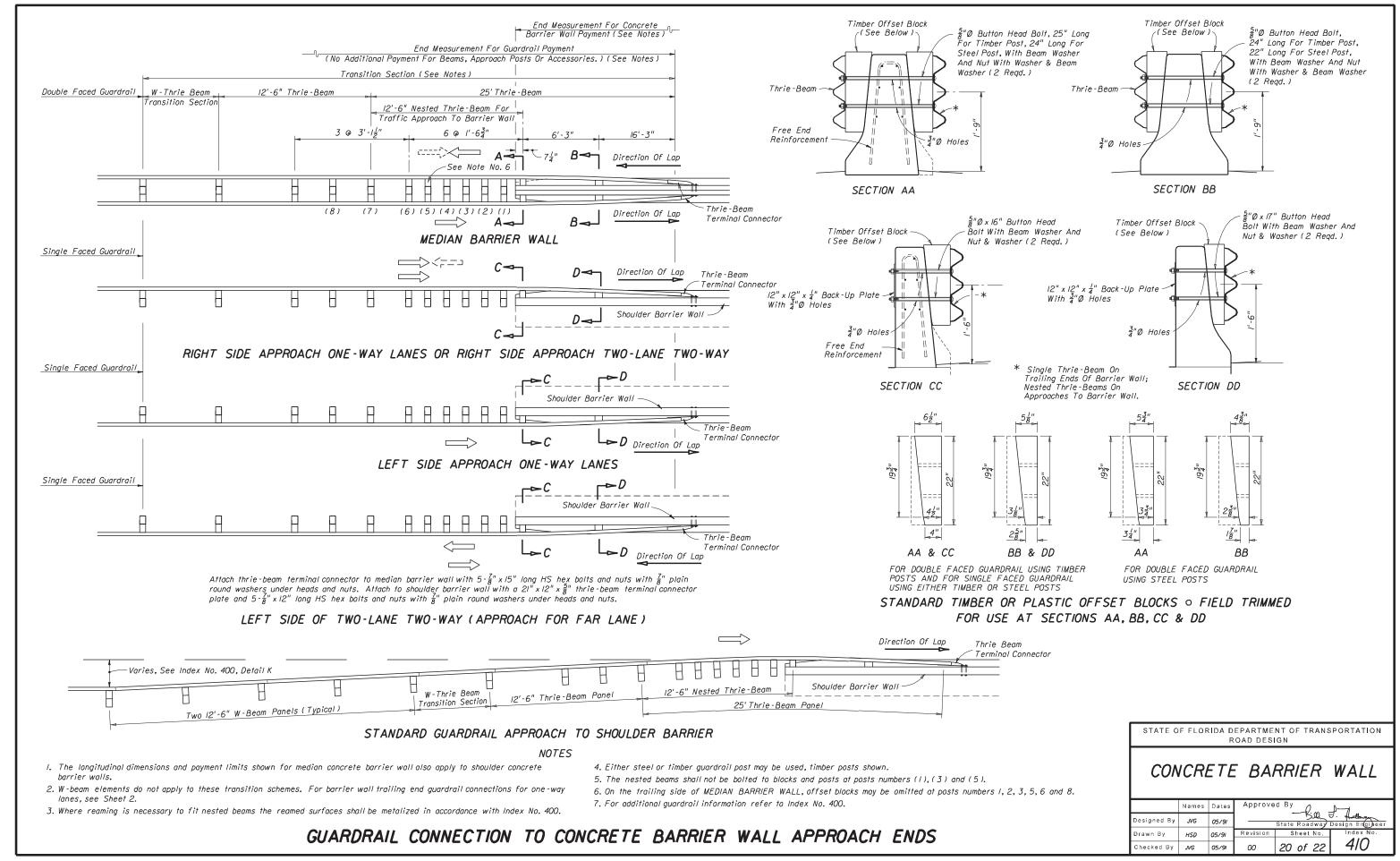
CONCRETE BARRIER WALL WHEN GUARDRAIL OFFSET FROM BENT OR PIER LESS THAN 3 FEET OR WHERE WALL STEM ABUTTS SUPPORTS OR PIER COLUMN

The details on this sheet are treatments to the F-shape concrete barrier walls depicted on Sheet Nos. 8 through 18, where site conditions impose reduced clearances between above ground hazards and the walls. Bridge bent supports and piers are shown. These treatments are not applicable to hazards that cannot provide lateral support for the walls. See the plans for limits of wall sections applied and other associated wall treatments.

> STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

CONCRETE BARRIER WALL

	Names	Dates	Approve	- 4/	D-71
Designed By	STAFF	10/97			Design Engineer
Drawn By	нкн	10/97	Revision	Sheet No.	Index No.
Checked By	JVG	10/97	00	19 of 22	410



GENERAL NOTES FOR TRAPEZOIDAL BARRIER WALL

- I. Concrete trapezoidal barrier wall can be either precast or cast in place. The wall is designed for zero deflection and shall have a minimum system length of 120'.
- 2. Where concrete trapezoidal barrier wall height changes from 42" to 48" or from 48" to 54", height change will be uniform for each 6" of height change per 90' of wall. Steel placement shall meet the dimensional positioning requirements of 42", 48" and 54" high barriers at the respective points along the vertical transition, with the vertical steel uniformly lengthened and the horizontal steel uniformly splayed throughout.
- 3. Welded wire fabric (WWF) made in accordance with ASTM A497 may be used as an option to the conventional reinforcement for precast or cast in place barrier wall, with the exception that only conventional reinforcement shall be used for horizontal transition and half wall sections. These sections shall be cast in place with length, shape and reinforcement as shown in this
- 4. To attain system length, precast segments shall be interconnected with rebar grids placed in the preformed slots and grouted into place. Segment length shall be not less than 30 unless otherwise specified in the plans.
- 5. The centerline axis of the barrier shall be vertical except where the roadway is superelevated in which case it shall be normal to the cross slope unless otherwise shown in the plans or directed by the Engineer.
- 6. For reflective barrier marker requirements see 'STANDARD BARRIER WALL SECTIONS' and the GENERAL NOTES, Sheet I.

8-#6 Bars, Bend To Obtain 2"

Bolt Circle Projection, Diameter And Length Per Pole Manufacturer's Specifications.

111.1

SECTION CC

30' Min. To Const.

Joint Each Side

Trapezoidal Barrier

Const. Joint Permitted

Cast In Place

(Each Side)

Note: For Additional Details See Sheet 4

LIGHT POLE MOUNTING

TOP VIEW

FRONT VIEW

€ Light Pole

Free End Reinforcement

Each Side

Junction Box

Or Pull Box

Clearance At Barrier Wall Footing

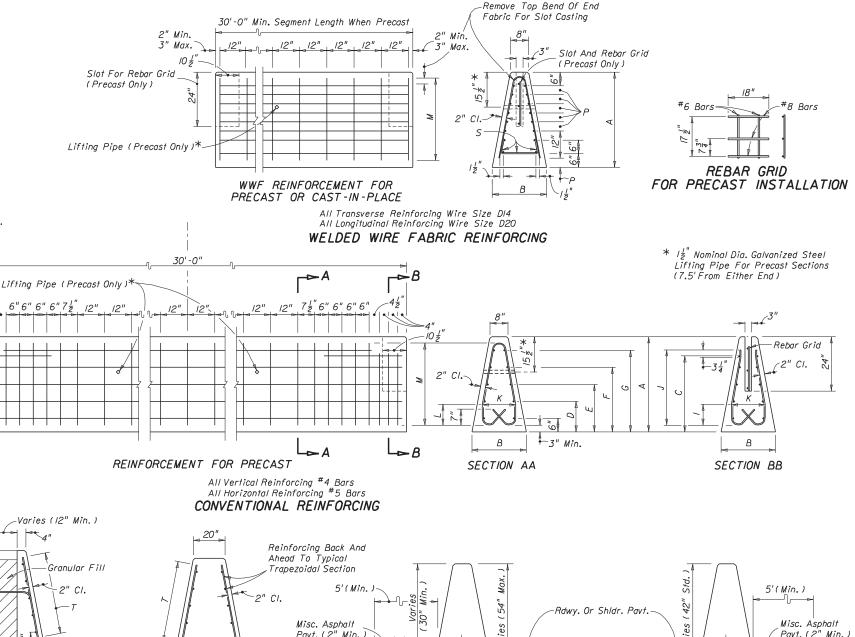
- 7. The concrete trapezoidal barrier wall is considered by the Federal Highway Administration to be innovative and may be used as such on Federal-Aid projects.
- 8. The concrete trapezoidal barrier wall is to be paid for under the contract unit price for Barrier Wall Concrete (Trapezoidal), LF. This price will include full payment for transitions, half walls, fill and concrete caps.

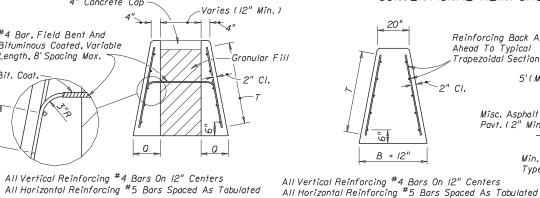
Light Pole

(Non-Frangible)

Recess Seat For

Light Pole Base





2" Min.

3" Max.

4" Concrete Cap

All Horizontal Reinforcing #5 Bars Spaced As Tabulated

TYPICAL HALF WALLS AROUND OBSTRUCTION

#4 Bar, Field Bent And

Length, 8' Spacing Max.

Bituminous Coated, Variable

REINFORCEMENT FOR CAST-IN-PLACE

Bit. Coat

SYSTEM LENGTH

B + 12"

SECTION AT BEGIN AND END

OF HALF WALLS

Payt. (2" Min.) Min. LBR 40 Type B Stabilization

Note: Foundation and paving options can be either or both sides of the wall; see the plans for the options applied. INDEPENDENT ROADWAY PROFILES

Min. Asphalt Pav

TYPICAL TRAPEZOIDAL SECTIONS

MATCHING ROADWAY PROFILES

Min. LBR 40

Type B Stabilization

For Transition Wall Plan See DETAIL I 'PLAN' TRANSITION SECTIONS

Barrier	DIMENSIONS (Inches)																	
Height (in.)	Α	В	С	D	Ε	F	G	Н	1	J	К	L	М	N	P	a	S	Т
42	42	24	33 ½	13 ½	21	28 ½	36	<i>1</i> 5	9½	33 4	/5	94	36	72	4	12	28	36
48	48	26 <u>9</u>	39 ½	15	24	33	42	17 <u>1</u>	10 3	39 🛓	17 ½	10 3	42	84	5	13 <u>9</u>	31 ½	42
54	54	28 9	45 ½	16 ½	27	37 ½	48	19 ½	12 1/4	45 🛔	19 ½	12 4	48	96	6	14 <u>9</u>	34 ¾	48

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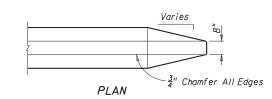
CONCRETE BARRIER WALL

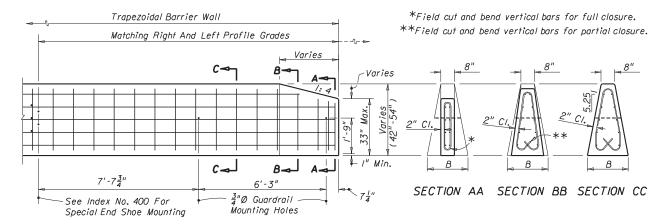
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Drawn By	HKH	11/93	Revision	Sheet No.	Index No.
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IN TRAPEZOIDAL SECTIONS

TRAPEZOIDAL BARRIER WALL

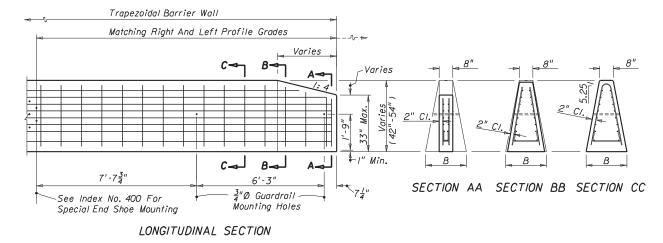
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LONGITUDINAL SECTION

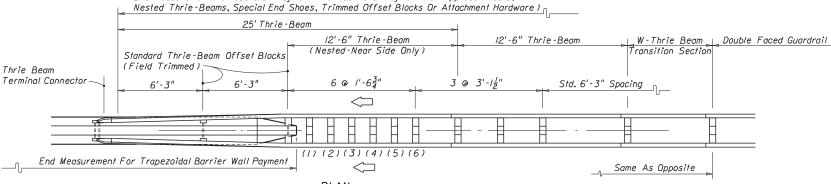
CONVENTIONAL REINFORCEMENT



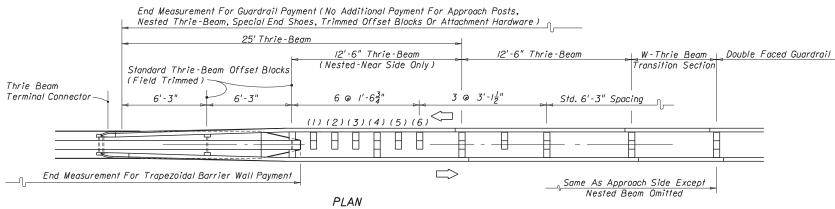
WELDED WIRE FABRIC REINFORCEMENT

END TREATMENT FOR PRECAST OR CAST-IN-PLACE WALLS

End Measurement For Guardrail Payment (No Additional Payment For Special End Shoes, Trimmed Offset Blocks Or Attachment Hardware) 25' W - Beam (Near And Far Side) Standard W - Beam Offset Blocks (Field Trimmed) 6'-3" Double Faced Guardrail (Std. 6'-3" Post Spacing) End Measurement For Trapezoidal Barrier Wall Payment (No Terminal Wall Bevel Required) PLAN End Measurement For Guardrail Payment (No Additional Payment For Approach Posts, Nested Thrie - Beams, Special End Shoes, Trimmed Offset Blocks Or Attachment Hardware)



PLAN **UNIDIRECTIONAL**



BIDIRECTIONAL

Note: Timber or steel posts may be used, timber posts shown.

GUARDRAIL TRANSITIONS AND CONNECTIONS

NOTES

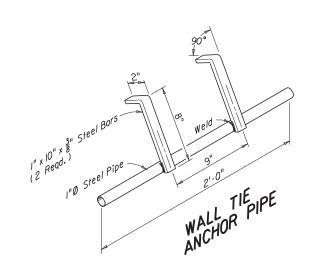
- I. Where reaming is necessary to fit nested beams the reamed surface shall be metalized in accordance with Index No. 400.
- 2. The nested beams shall not be bolted to the posts and blocks at post numbers (1), (3) and (5).
- 3. For additional wall details, see Sheet 21.
- 4. For additional guardrail information refer to Index No. 400.

GUARDRAIL CONNECTION TO TRAPEZOIDAL BARRIER WALL

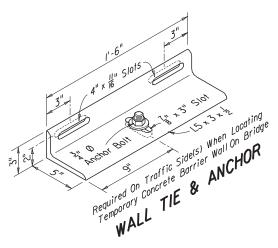
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

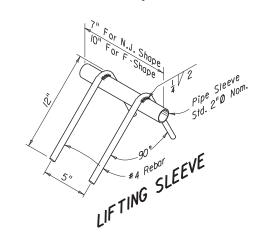
CONCRETE BARRIER WALL

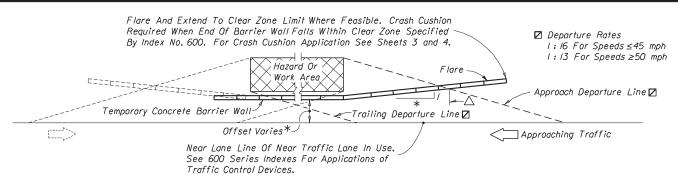
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ı	Designed By	JVG/HKH	7/96		State Roadway	Design Englineer
	Drawn By	нкн	7/96	Revision	Sheet No.	Index No.
	Checked By	JVG	7/96	00	22 of 22	410





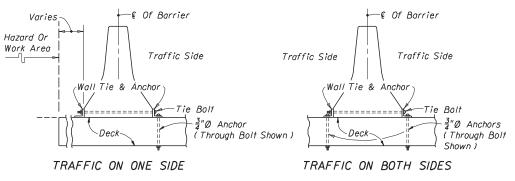






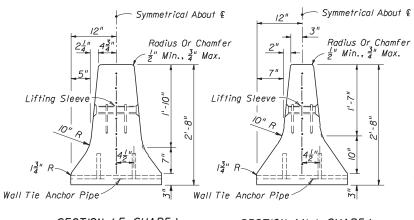
- △ The length of temporary concrete barrier wall is determined by the intersect point between the departure lines and the traffic side toe of wall. The approach departure line location is determined by the line intersect with the back of the hazard, and, the trailing departure line location is determined by the line intersect with the front side of the trailing end of the hazard. For unanchored approach and trailing ends of temporary concrete barrier walls with standard length units, a minimum of two and one-half $(2\frac{1}{2})$ units is required outside the length of need to provide wall end anchorage. Where a redirective crash cushion is used to shield an approach end of a temporary concrete barrier wall, the crash cushion may be located by the departure line intersect point indicated on the standard drawing for the crash cushion used, the wall beginning unit will be positioned relative to the crash cushion position and the beginning unit anchored; and, interconnections between the end unit and crash cushion made as required for the specific crash cushion type.
- * The wall offset from the near traffic lane, wall flare rate and wall flare length are to be in accordance with the alignment called for in the plans and the alignments called for by other department Roadway And Traffic Design Standards specified in the plans; in absence of either plan requirement, the offset shall be as determined by the Engineer, and, unless other flare rates are approved by the Engineer the flare rates to be applied are I:10 or flatter for speeds ≤45 mph and I:15 or flatter for speeds ≥50 mph; see Index No. 600 for other flare rates on expressway

TEMPORARY CONCRETE BARRIER WALL ALIGNMENT

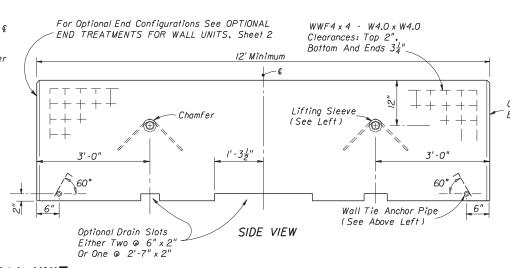


Anchor bolts shall have a pullout and shear capacity of 14,000 lbs. Expansion or chemical anchor bolts will be used to secure walls to approach slabs. Expansion or chemical anchor bolts or through bolts with washers and nuts will be used to secure walls to bridge decks. Core drills shall be used to construct through bolt holes, and, drills specified by the manufacturer shall be used to construct expansion and chemical anchor bolt holes. Chemical anchorage shall be an Adhesive Material System in accordance with Specification Sections 4l6 and 937. After removal of walls, anchors shall be removed to I" min. below deck surface and holes filled with epoxy grout.

BRIDGE DECK AND APPROACH SLAB INSTALLATIONS







Anchor And Washer

Wall Tie & Anchor

- Wall Tie & P OR OBECK ON MOUN BRIDGE SLAB VIEW BRIDGETORIAL APPROJECTORIAL

WALL UNIT

GENERAL NOTES

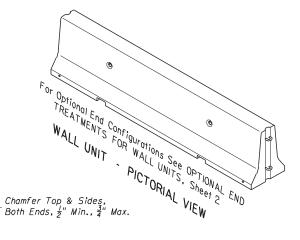
- I. Temporary concrete barrier wall units may be either the New Jersey shape or the F-Shape configuration, unless the plans specify other types of temporary concrete barrier wall; however, intermixing of units with different shapes in a continuous run of barrier will not be permitted.
- 2. Material and workmanship for the wall shall meet the requirements of Sections 400 and 521 of the Standard Specifications, except the bottom of the unit can be finished to a dense uniform surface by floating in lieu of the Class 3 finish. Concrete shall be
- 3. Type C Steady-Burn Lights are to be mounted on top of temporary concrete barrier walls that are used as barriers along travel ways in work zones. The lights are to be spaced at 50' centers in transitions, 100' centers on curves and 200' centers on tangent roadways. For additional information refer to 'Warning Lights' on Index No. 600.
- 4. Wall units shall not be used for permanent barrier wall construction regardless of unit length, unless specifically permitted by the plans.
- 5. The temporary concrete barrier wall units with the optional end connections shown on this index are the standard optional units for Florida Department Of Transportation projects. Standard optional end units can be intermixed in a run of wall, and interconnected with other barrier systems as specified on other standard drawings or with appropriate transitions as detailed in the plans.

Temporary concrete barrier wall units with end configurations that are on the 'Qualified Products' listing may be substituted for the standard optional end units when approved by the Engineer: however, substitution units cannot be intermixed with dissimilar units in a run of wall. Substitution units shall have positive interconnections between each adjoining unit; wall units with plain ends will not be permitted regardless of ties or anchorages.

- 6. Wall units may be reused provided they have the structural integrity and surface qualities of new units.
- 7. Wall units shall be furnished by the Contractor except when the plans stipulate the availability of Department owned units. Regardless of unit source the Contractor shall furnish all hardware and shall be responsible for all handling including loading, transport, unloading, stockpiling, installation, removal and return.

When the plans stipulate that temporary concrete barrier wall(s) are to become property of the Department at completion of the project, only the standard optional end treatment units are to be used, except as otherwise specified in the plans.

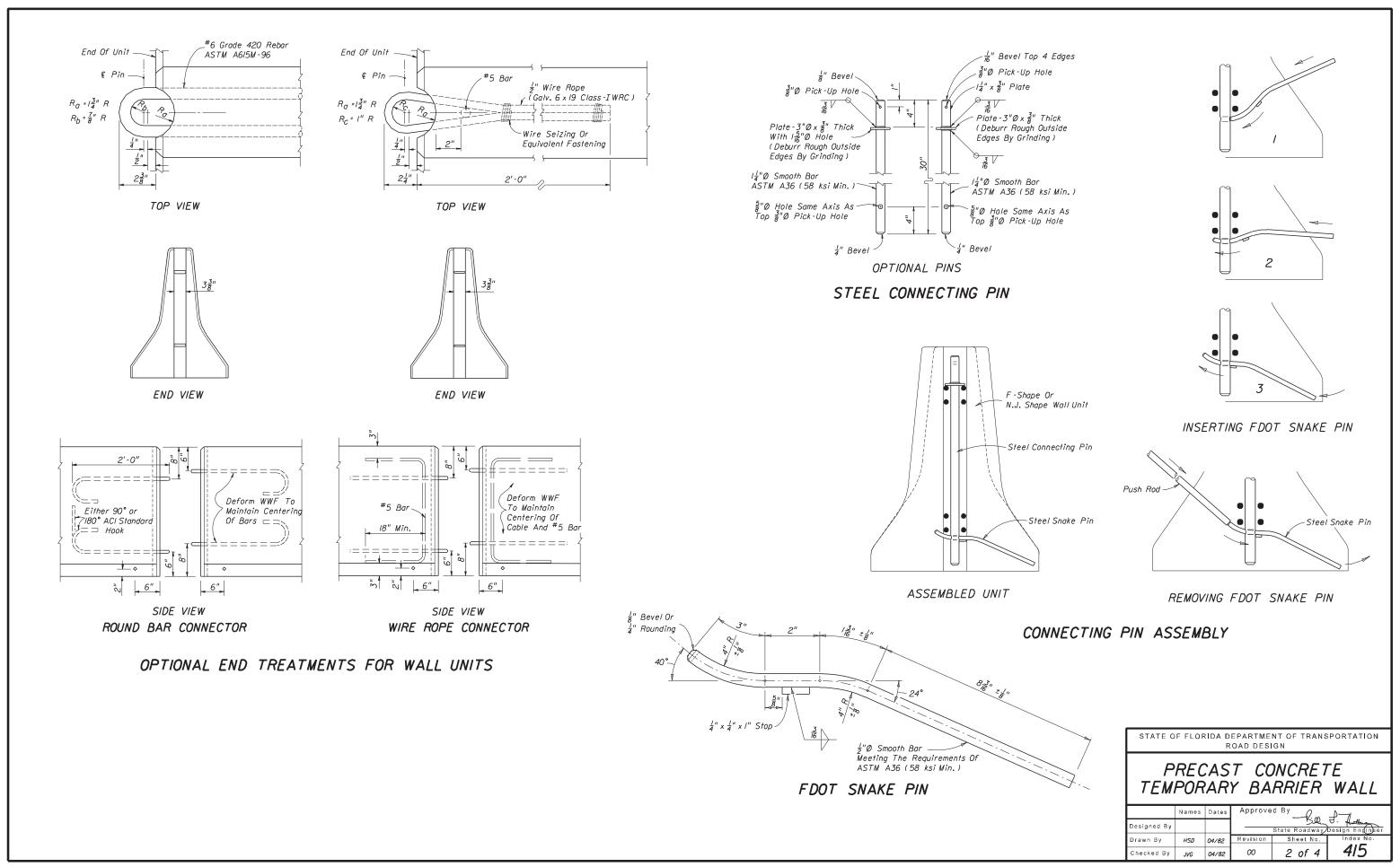
- 8. If the plans specify Barrier (Temporary) Optional, the Contractor has the option to furnish either concrete or water filled barriers. If the plans specify Barrier (Temporary) Concrete, substitution with water filled barriers will not be permitted.
- 9. Wall units used for work zone traffic control and other temporary applications shall be paid for under the contract unit price for Barrier (Temporary) Concrete, LF, or Barrier (Temporary) Optional, LF. Type C Steady-Burn Lights shall be paid for under the contract unit price for Lights, Temp. Barrier Wall Mount (Type C, Steady-Burn), ED.

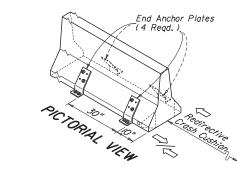


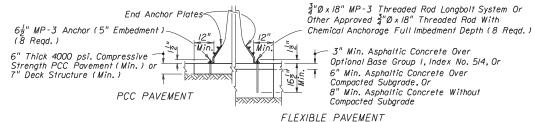
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

PRECAST CONCRETE TEMPORARY BARRIER WALL

	Names	Dates	Approve	d By	
Designed By				State Roadway	Design Engineer
Drawn By	HSD	04/82	Revision	Sheet No.	Index No.
Checked By	JVG	04/82	00	I of 4	415







SURFACE ANCHORAGE REQUIREMENTS

END ANCHORAGE NOTES

- I. For temporary barrier wall end anchorage applications, see 'TEMPORARY CONCRETE BARRIER WALL ALIGNMENT' and 'NOTES FOR TEMPORARY CONCRETE BARRIER WALL END SHIELDING'.
- 2. The temporary concrete barrier wall anchor plate depicted above is a proprietary design by Energy Absorption Systems, Inc. Other temporary anchorage methods can be substituted when wall rigidity is assured by any of the following:
 - (a) proven by associated crash test of redirective crash cushions, or
 - (b) meet anchorage prescribed in 'A Guide To Standardized Highway Barrier Hardware', or
 - (c) crash cushion manufacturer's engineered design, or
 - (d) approved shop drawings on a case by case basis.
- 3. The cost for anchoring the wall segment will be included in the cost for the adjoining redirective crash cushion.

BARRIER WALL END ANCHORAGE

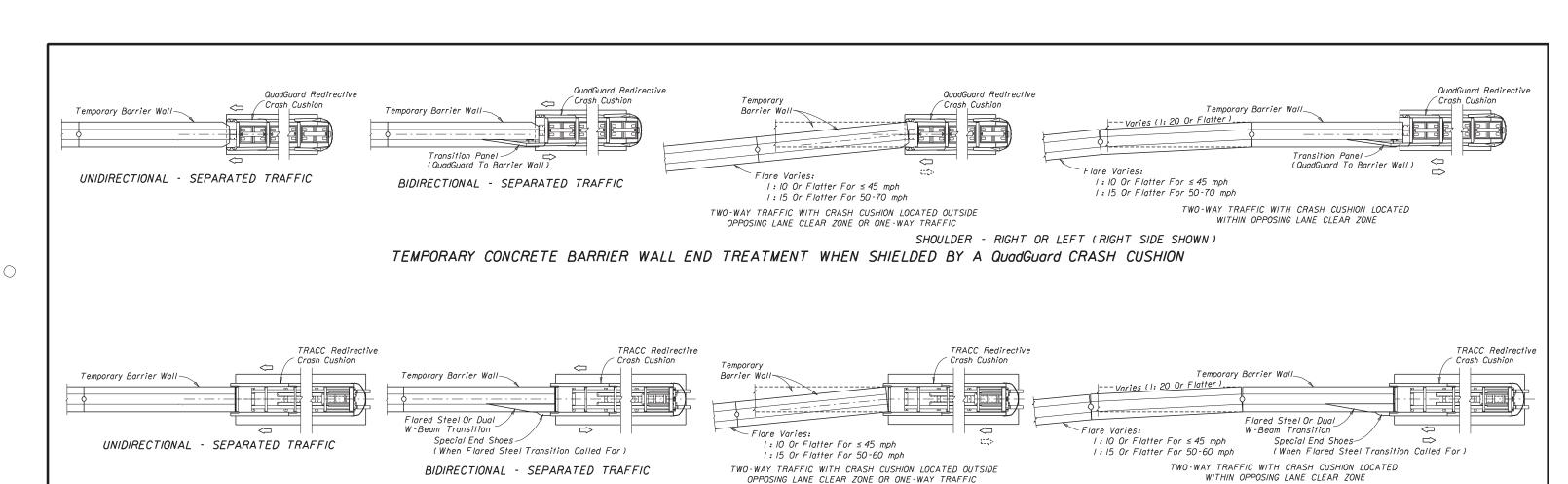
NOTES FOR TEMPORARY CONCRETE BARRIER WALL END SHIELDING

- I. Redirective crash cushions are the principal (standard) device to be used for shielding approach ends of temporary concrete barrier walls. Except where the plans designate a particular type of redirective crash cushion for a specific location, the contractor has the option to construct either the REACT 350, QuadGuard, ADIEM 350 or TRACC crash cushions subject to the uses and limitations described on Index Nos. 434, 435, 436 and 440 respectively. The barrier wall end segment must be anchored to a paved surface in accordance with 'BARRIER WALL END ANCHORAGE'.
- 2. Temporary redirective crash cushions shall be installed in accordance with the manufacturer's specifications and recommendations. Temporary crash cushions can be either new or functionally sound used devices. Performance of intended function is the only condition for acceptance, whether the crash cushion is new, used, refurbished, purchased, leased, rented, on loan, shared between projects, or made up of mixed new and used components.
- 3. Inertial crash cushions are not optional systems for locations designated for redirective crash cushions by the plans; can not be substituted for redirective crash cushions without expressed approval by the Engineer; and, such substitutions are not eligible for VECP consideration.
- 4. A yellow post mounted Type I Object Marker shall be centered 3' in front of the nose of all temporary crash cushions. Mounting hardware shall be in accordance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost
- 5. Optional temporary redirective crash cushions are to be paid for per location under the contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

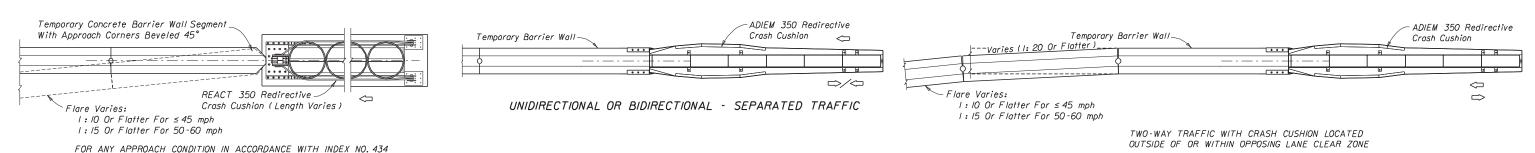
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

PRECAST CONCRETE TEMPORARY BARRIER WALL

	Names	Dates	Approve	d By	D—1
Designed By				State Roadway	Design Engineer
Drawn By	нкн	03/99	Revision	Sheet No.	Index No.
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SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)
TEMPORARY CONCRETE BARRIER WALL END TREATMENT WHEN SHIELDED BY A TRACC CRASH CUSHION



TEMPORARY CONCRETE BARRIER WALL END TREATMENT WHEN SHIELDED BY A REACT 350 CRASH CUSHION

TEMPORARY CONCRETE BARRIER WALL END TREATMENT WHEN SHIELDED BY AN ADIEM 350 CRASH CUSHION

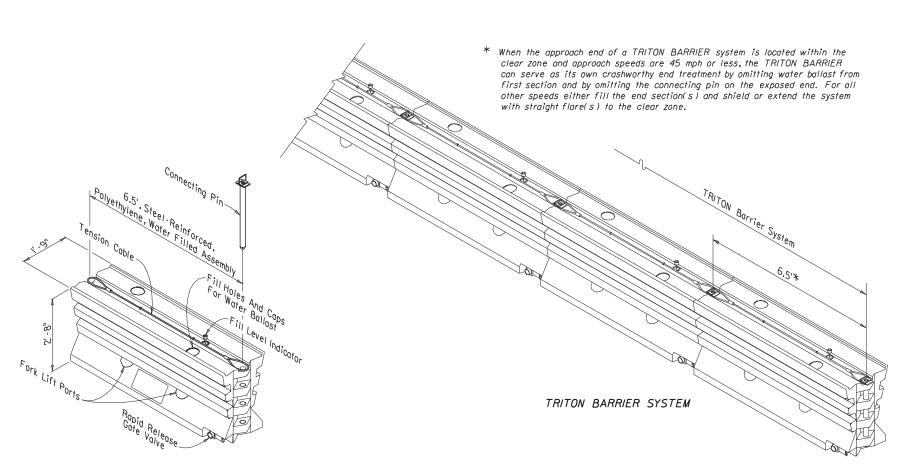
See 'TEMPORARY CONCRETE BARRIER WALL ALIGNMENT', 'BARRIER WALL END ANCHORAGE' and 'NOTES FOR TEMPORARY CONCRETE BARRIER WALL END SHIELDING' for additional information.

SHIELDING TEMPORARY CONCRETE BARRIER WALL ENDS WITH REDIRECTIVE CRASH CUSHIONS (REDIRECTIVE OPTION)

PRECAST CONCRETE
TEMPORARY BARRIER WALL

SHOULDER - RIGHT OR LEFT (RIGHT SIDE SHOWN)

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TRITON BARRIER SECTION

SUPPLEMENTAL GENERAL NOTES FOR THE TRITON BARRIER

- I. The system presented on this standard drawing (index) under the label TRITON BARRIER is a proprietary design by Energy Absorption Systems, Inc. and is marketed under the trade name TRITON BARRIER.
- 2. This index provides the general graphics and information necessary to field identify component parts of the TRITON BARRIER and their incorporation as a whole system for Department standard applications.
- 3. The TRITON BARRIER system can be installed as a free standing system or in combination with other Department temporary and permanent barrier systems, exclusive of other proprietary water filled barrier systems.
- 4. Connections between the TRITON BARRIER and other barrier systems shall be as shown in the 'TRITON BARRIER TRANSITION HARDWARE ASSEMBLIES'. Variation from these connections shall be as detailed in the plans or as prescribed by the manufacturer.
- 5. The TRITON BARRIER section or sections are not to be used as perpendicular road closure blocks, whether connected, unconnected, filled or unfilled.
- 6. Sections shall be installed in alternating white and work zone safety orange colors.
- 7. The TRITON BARRIER systems shall be paid for under the contract unit price for Barrier (Temporary) (Water Filled), LF, or Barrier (Temporary) (Optional), LF and shall be full compensation for furnishing and installing TRITON BARRIER in accordance with this index, with the plans and with the manufacturers detailed drawings, procedures and specifications. The cost for transition hardware detailed in this index shall be included in the contract unit price for the barrier. TRITON modules considered a part of the systems crashworthy end treatment shall be included in the linear measure; other crashworthy end terminals, crash cushions or other shielding required for use of the TRITON barrier will not be included in the contract unit price for the barrier.

SUPPLEMENTAL DESIGN NOTES AND GUIDELINES FOR THE TRITON BARRIER

- I. The longitudinal system can be used for work zone speeds of 60 mph or less. Transition hardware can be used in areas where speeds are limited to 45 mph or less.
- Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the TRITON BARRIER, and until such alternatives are available, the TRITON BARRIER need not be bid against other proprietary items.

TRITON BARRIER

GENERAL NOTES

- I. This standard drawing (index) presents proprietary temporary water filled barrier designs and is produced by the Florida Department Of Transportation solely for use by the Department and its assignees.
- 2. Any system presented on this index can be used as a temporary barrier in traffic control work zones and other Department permitted traffic control zones but cannot be constructed as a permanent barrier.
- 3. All systems shall be assembled and installed in accordance with the manufacturer's detailed drawings, procedures and specifications; however, installation will be limited to the applications shown on this index, except when otherwise detailed in the plans or approved by shop drawings or approved by the Engineer.
- 4. Water filled barrier systems are to be used only as longitudinal systems. A longitudinal system may include encapsulating work space barriers within low speed intersections only where the approach longitudinal system deflects the traffic alignment around the work space enclosure.
- 5. One type proprietary water filled barrier system is not to be used in conjunction with another type proprietary water filled barrier system, except when specifically called for and detailed in the plans.
- 6. All water filled barrier system sections shall be interconnected with manufacturer and Department approved crash tested connections, i.e., no individual sections or interconnected sections of substandard length are to stand alone, except when specifically called for and detailed in the plans, or for specific applications of interconnectioned sections around work spaces shown on this index.
- 7. Water filled barrier systems are not to be used on surfaces with cross slopes exceeding 0.05 (steeper than 1:20), including the surface within the design deflection space behind the barrier.
- 8. Water filled barrier systems are not to be used on grades steeper than 5%, nor placed over surface irregularities that cause vertical deflection exceeding 1:20 between connected sections.
- 9. Water filled barrier systems are not permitted on bridges or approach slabs; however, they can be placed over box culverts, including those of bridge length, where design deflection space is adequate. The system should be used on concrete pavements only where the Engineer determines that the dynamic loading of pavement slabs will not cause the system to crab out of alignment.
- 10. Temporary water filled barriers are to be paid for under the contract unit price for Barrier (Temporary) (Water Filled), LF, or Barrier (Temporary) (Optional), LF. If the plans specify Barrier (Temporary) (Optional), the Contractor has the option to furnish either concrete or water filled barriers. If the plans specify Barrier (Temporary) Water Filled, substitution with concrete barriers will not be permitted. For additional payment information see the supplemental general notes for the individual barrier systems.

Type C Steady-Burn lights are to be mounted on top of all water filled barriers used along travelways in work zones. The lights are to be spaced at 50' centers on transitions, 100' centers on curves and 200' centers on tangent roadways. Lights shall be paid for under the contract unit price for Lights (Temporary Barrier Wall Mount) (Type C Steady-Burn), ED.

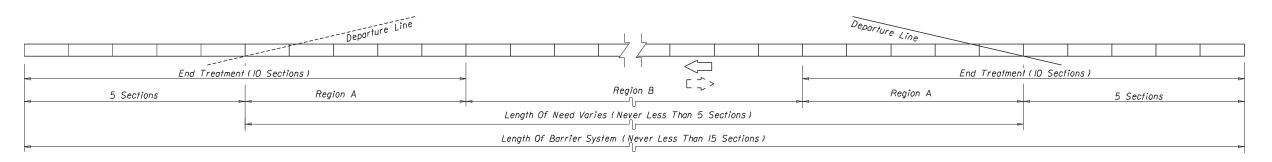
DESIGN NOTES

I. The TRITON and GUARDIAN water filled barriers are considered by the Federal Highway Administration to be innovative temporary barriers, and, may be used as such toward compliance with the percentage of innovative barrier required in the total median barrier on Federal Aid Projects.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TEMPORARY WATER FILLED BARRIERS

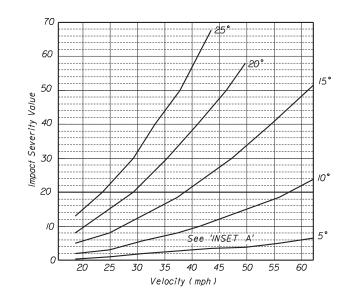
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Designed By	MFG/HKH	6/95		State Roadway	Design Engineer
Drawn By	нкн	6/95	Revision	Sheet No.	Index No.
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Note: For Departure Line requirements see Index No. 400.

When TRITON BARRIER is used as its own end treatment fill all sections with water ballast except the approach end section(s). Do not use connecting pin on the exposed end of the end section(s).

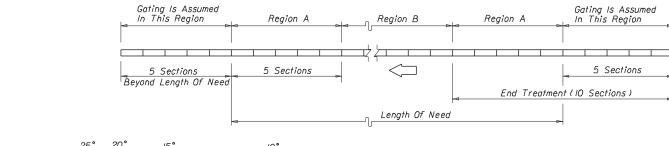
SYSTEM LENGTHS FOR UNIDIRECTIONAL OR BIDIRECTIONAL TRAFFIC

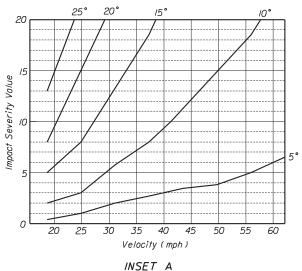


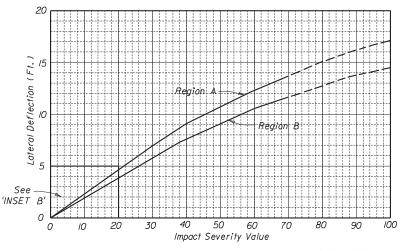
DETERMINING THE IMPACT ANGLE CURVE TO APPLY

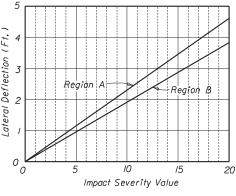
Except where the plans call for the use of a certain impact angle curve, or where a certain impact angle is anticipated by site specific conditions, the impact angle curve to be used in determining impact severity will be selected on the following basis:

Barrier Location	Graph Curve
Parallel to tangent roadway	5°
Parallel to and on the inside of roadway curve	5°
Standard lane shift or drop (WS & $\frac{WS}{60}^2$)	5°
Parallel to and on the outside of roadway curve	5° (10°)[15°]
Approach flared end section on inside of roadway curve	10°
Approach flared end section on approach tangent roadway	10°
Approach flared end section on outside of roadway curve	10° (15°)[25
() Max. Curvature (Min. Radius), High Speed Facilities	
[] Max. Curvature (Min. Radius), Low Speed Facilities	









INSET B

Notes: Curves for Regions 'A' and 'B' apply to vehicles ≤ 4400 lbs.

— — Indicates impact severity levels created by higher impact angles not anticipated in work zone.

SINGLE ROW TRITON BARRIER INSTALLATION DEFLECTION CURVES

IMPACT SEVERITY DETERMINATION FOR VEHICLES
≤4400 LB IMPACTING SINGLE ROW TRITON SYSTEM

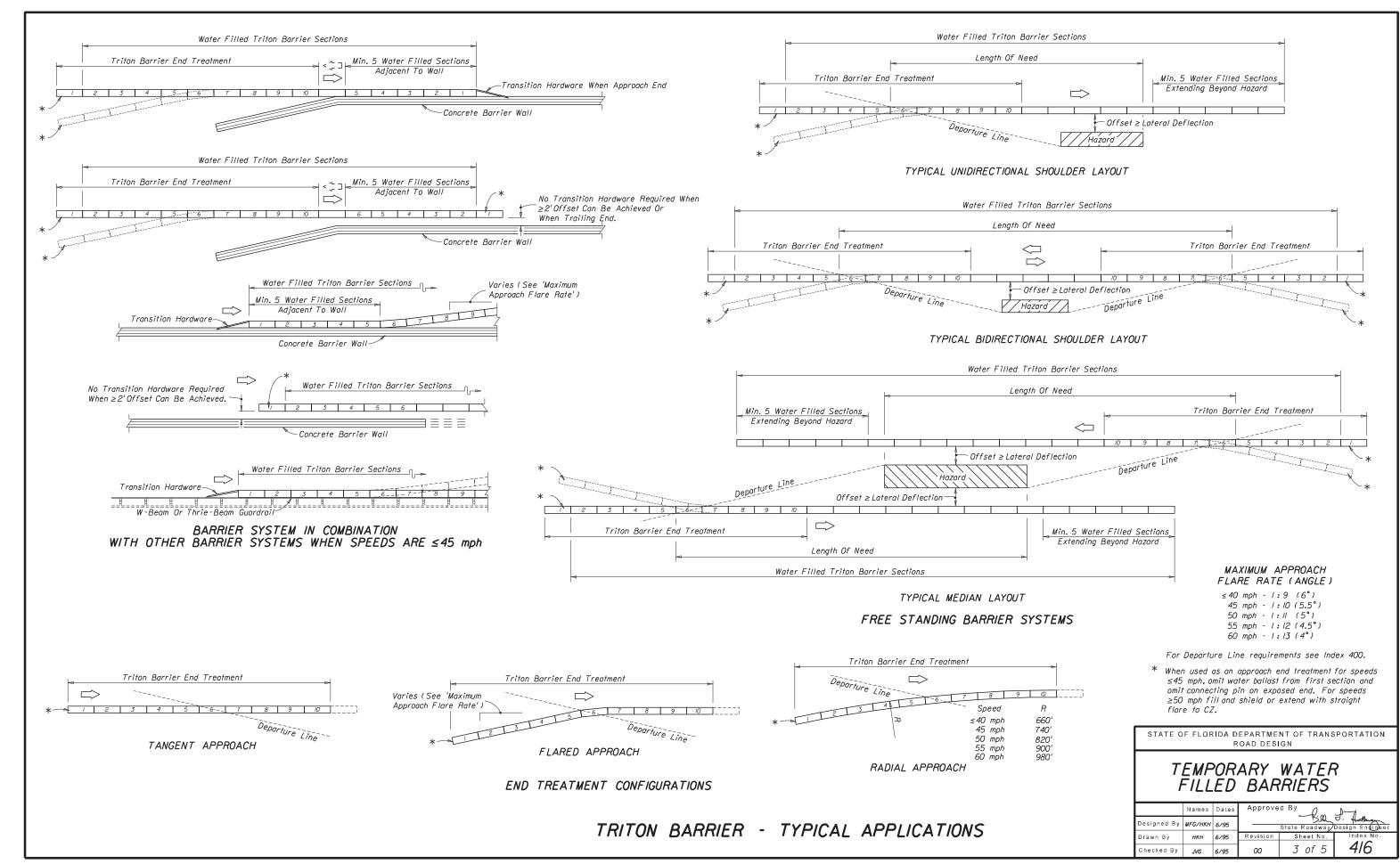
IMPACT SEVERITY AND LATERAL DEFLECTION DISTANCES

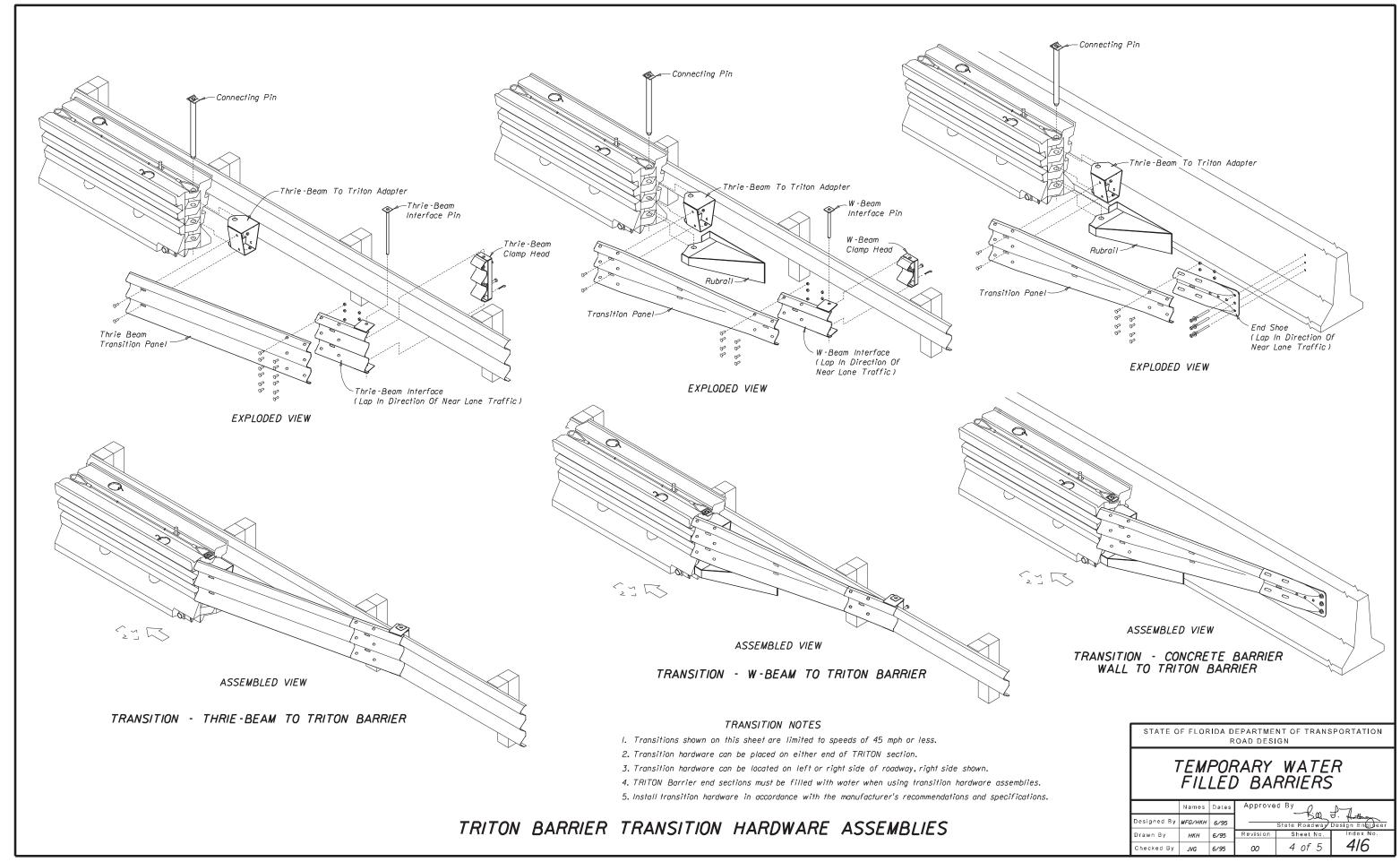
TRITON BARRIER SYSTEM LENGTHS AND DEFLECTIONS

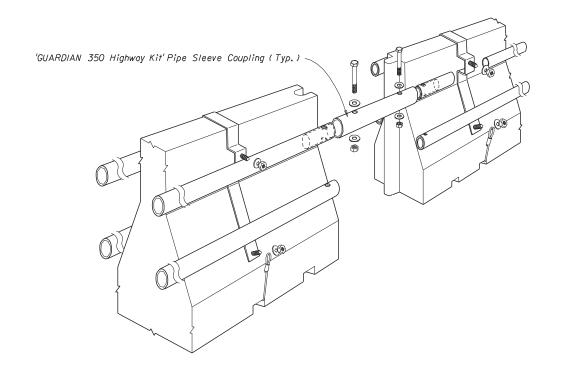
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TEMPORARY WATER FILLED BARRIERS

	Names	Dates	Approve	d By	A-71
Designed By	MFG/HKH	6/95		State Roadway/	Design Engineer
Drawn By	нкн	6/95	Revision	Sheet No.	Index No.
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SUPPLEMENTAL GENERAL NOTES FOR THE GUARDIAN BARRIER

- I. The barrier units presented on this standard drawing (index) and the label GUARDIAN are proprietary designs by Safety Barrier Systems and are marketed under the trade name GUARDIAN Safety Barrier.
- 2. This index provides general schematics and information necessary to field identify the water filled polyethylene segmental barrier module and the module frame and basic connections, but does not identify the incorporation of the modules and frame connections into a whole system. Any use of the GUARDIAN must be in accordance with the details on the plans, or by shop drawing approval or by the Engineer in absence of plan detail.
- 3. The GUARDIAN modules are approved for use on highways with all design speeds and only when the "GUARDIAN 350 Highway Kit" is incorporated throughout the system in use.
- 4. The GUARDIAN modules can be used only in a stand alone system. i.e., not connected to other types of barrier systems.
- 5. The GUARDIAN can be used only as a longitudinal barrier on the State maintained highway system. Any longitudinal system must have a minimum of eleven (II) longitudinally connected modules in advance of and following the length of need; in no case can the longitudinal run of barrier be less than 33 modules.

The approach end of the GUARDIAN must either extend to the outer limit of the clear zone; be shielded by a crash cushion; or, begin behind but not connected to another barrier or shielding feature.

6. The GUARDIAN system must be placed on a cross slope not exceeding I: IO, and located to provide a deflection distance between the system and hazards in accordance with the table below.

		N BARRIER WITH 350 HIGHWAY KIT TED BARRIER DEFLECTION (FEET)							
Vehicle Vehicle Impact Angle (Degrees) Speed									
	(mph)	25°	20°	15°	10°	5°			
	<i>≤</i> 45	6.5	5.3	4.0	2.7	1.3			
	50	8.0	6.4	4.9	3.3	1.6			
	55	9.5	7.7	5.8	4.0	2.0			
	60	//.2*	9.0	6.9	4.6	2.3			
	* Observed Value (Crash Test Result) Other Values Manufacturers Calculated Estimates								

7. The GUARDIAN barrier system shall be paid for under the contract unit price for Barrier (Temporary) (Water Filled), LF, or Barrier (Temporary) (Optional), LF, and shall be full compensation for furnishing and installing GUARDIAN barrier in accordance with this index, with the plans and with the manufacturer's detailed drawings, procedures and specifications. Any crashworthy end terminal, crash cushion or other shielding required for use of the GUARDIAN barrier will not be included in the contract unit price for the barrier.

SUPPLEMENTAL DESIGN NOTES FOR THE GUARDIAN BARRIER

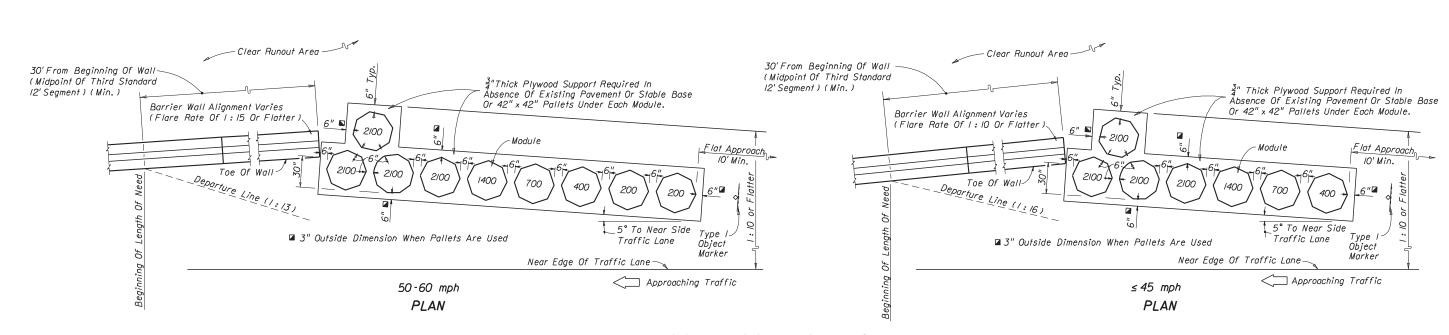
- I. At time of publication of this standard no crash test data was available to provide a crashworthy end terminal design using the barrier modules; only the requirement for eleven (II) interconnected modules preceding and following the length of need, based on available crash test data.
- 2. Systems included in any maintenance of traffic plan will require detailed location and placement information.
- 3. Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the GUARDIAN barrier, and until such alternatives are available, the GUARDIAN barrier need not be bid against other proprietary items.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TEMPORARY WATER FILLED BARRIERS

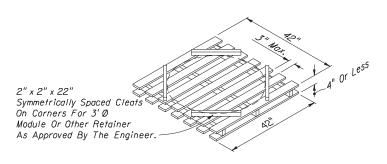
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GUARDIAN BARRIER WITH 350 HIGHWAY KIT



Note: Numbers shown inside modules indicate mass in pounds of sand. All modules are approximately 3' in diameter with heights ranging from 3' to 3'-9".

INERTIAL CRASH CUSHION ARRAYS



Pallet Shall Be Constructed Of Wood Or Other Frangible Or Resilient Materials Other Than Metals, And, Shall Be Sufficiently Durable To Support Modules For Their Expected Period Of Use; Wood Pallet Detail Shown.

INERTIAL MODULE PALLET

NOTES FOR TEMPORARY INERTIAL CRASH CUSHIONS

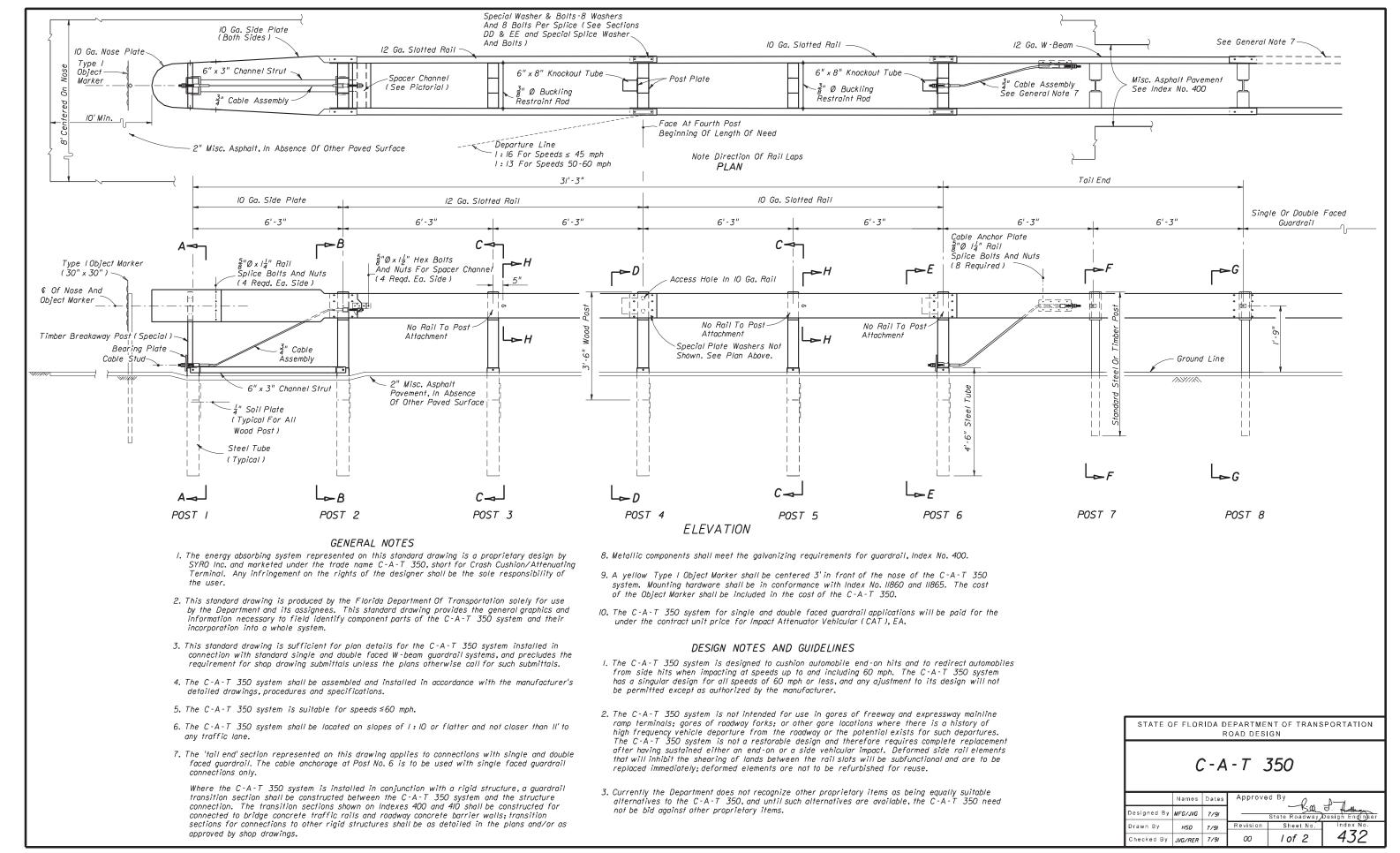
- I. The crash cushion arrays shown on this index can be used on the State highway system only for shielding temporary concrete barrier wall approach ends (a) where called for by the plans, or (b) where the use of redirective crash cushions is declared infeasible by the Engineer. Any permanent use of inertial modules will require site specific design. These arrays can not be substituted for redirective crash cushions called for in the plans without expressed approval by the Engineer; and, such approved substitutions are not eligible for VECP considerations.
- 2. Inertial crash cushions are gating type crash cushions, and a clear runout area back of the array must be provided. The arrays shown can be used for outer roadway applications, exclusive of gore areas, and for median applications where the median width is sufficient to provide clear zone width between the back side module and the near lane of the opposing traffic.
- 3. Inertial crash cushion modules shall be installed in accordance with the manufacturer's specifications and recommendations, and can be constructed of either new or functionally sound used modules.
- 4. Anchorage of barrier wall end segment is not required.
- 5. A yellow post mounted Type I Object Marker shall be centered 3' in front of the nose of all crash cushion arrays. Mounting hardware shall be in accordance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost of the modules.
- 6. Temporary inertial crash cushions are to be paid for per module under the contract unit price for Impact Attenuator Modules (Inertial) (Temporary), EA.

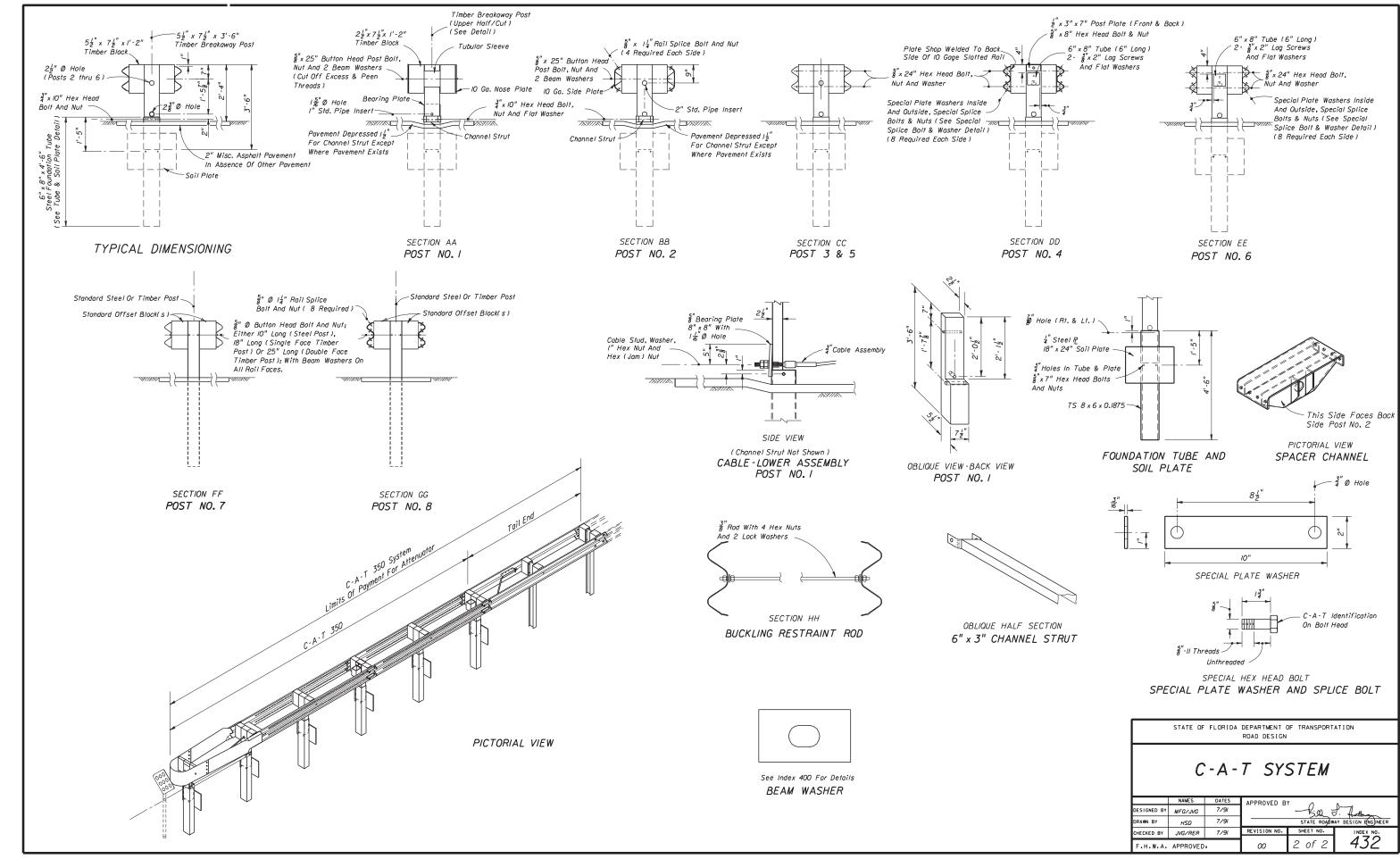
TEMPORARY INERTIAL CRASH CUSHIONS FOR SHIELDING ENDS OF TEMPORARY CONCRETE BARRIER WALL

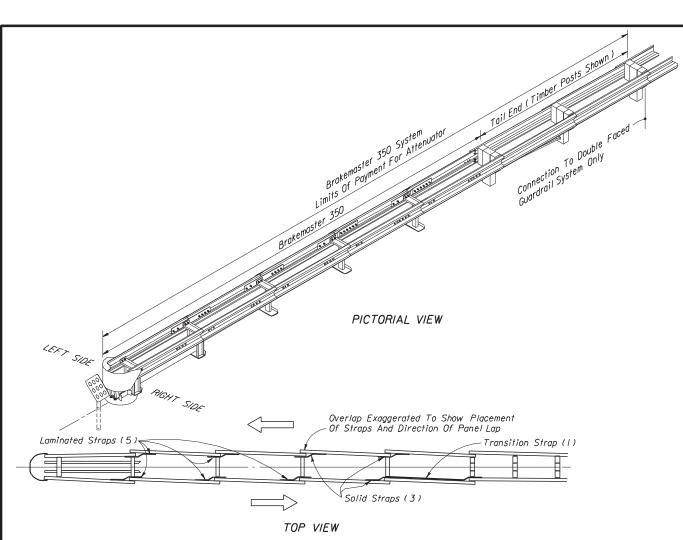
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

INERTIAL CRASH CUSHION

	Names	Dates	Approve	d By	_A-7
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Drawn By	HKH	3/99	Revision	Sheet No.	Index No.
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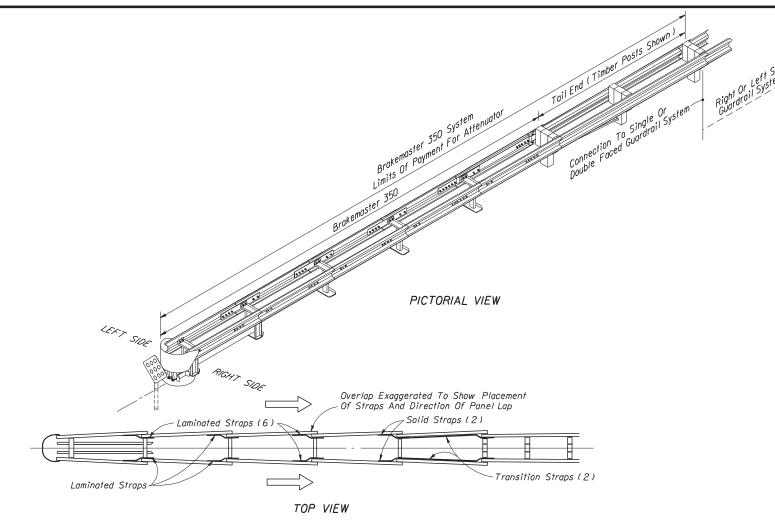




BIDIRECTIONAL SYSTEM

GENERAL NOTES

- I. The energy absorbing system represented on this standard drawing is a proprietary design by Energy Absorption Systems, Inc. and marketed under the trade name Brakemaster 350. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for use by the Department and its assignees. This standard drawing provides the general information and graphics necessary to field identify component parts of the Brakemaster 350 system and their incorporation into a whole system.
- 3. This standard drawing is sufficient for plan details for the Brakemaster 350 system installed in connection with standard single and double faced W-beam guardrail systems, and precludes the requirement for shop drawing submittals unless the plans otherwise call for such submittals.
- 4. The Brakemaster 350 system shall be assembled and installed in accordance with the manufacturer's detailed drawings, procedures and specifications.
- 5. The Brakemaster 350 system is suitable for speeds ≤ 60 mph.
- 6. The Brakemaster 350 system shall be located on slopes of I: IO or flatter and not closer than II' to any traffic lane.
- 7. The 'tail end' section represented on this drawing applies to connections with single and double faced guardrail. Where the Brakemaster 350 system is installed in conjunction with safety shaped or vertical faced barrier walls or other rigid structures, a special transitional guardrail section between the Brakemaster 350 and wall or structure shall be as detailed on Index No. 410 or as approved by shop drawings.
- 8. Metallic components shall meet the galvanizing requirements for guardrail, Index No. 400.
- 9. A yellow Type I Object Marker shall be centered 3' in front of the nose of the Brakemaster 350 system. Mounting hardware shall be in conformance with Index No. II860 and II865. The cost of the Object Marker shall be included in the cost of the BRAKEMASTER 350.
- 10. The Brakemaster 350 system will be paid for under the contract unit price for Impact Attenuator Vehicular (Brakemaster), EA.



UNIDIRECTIONAL SYSTEM

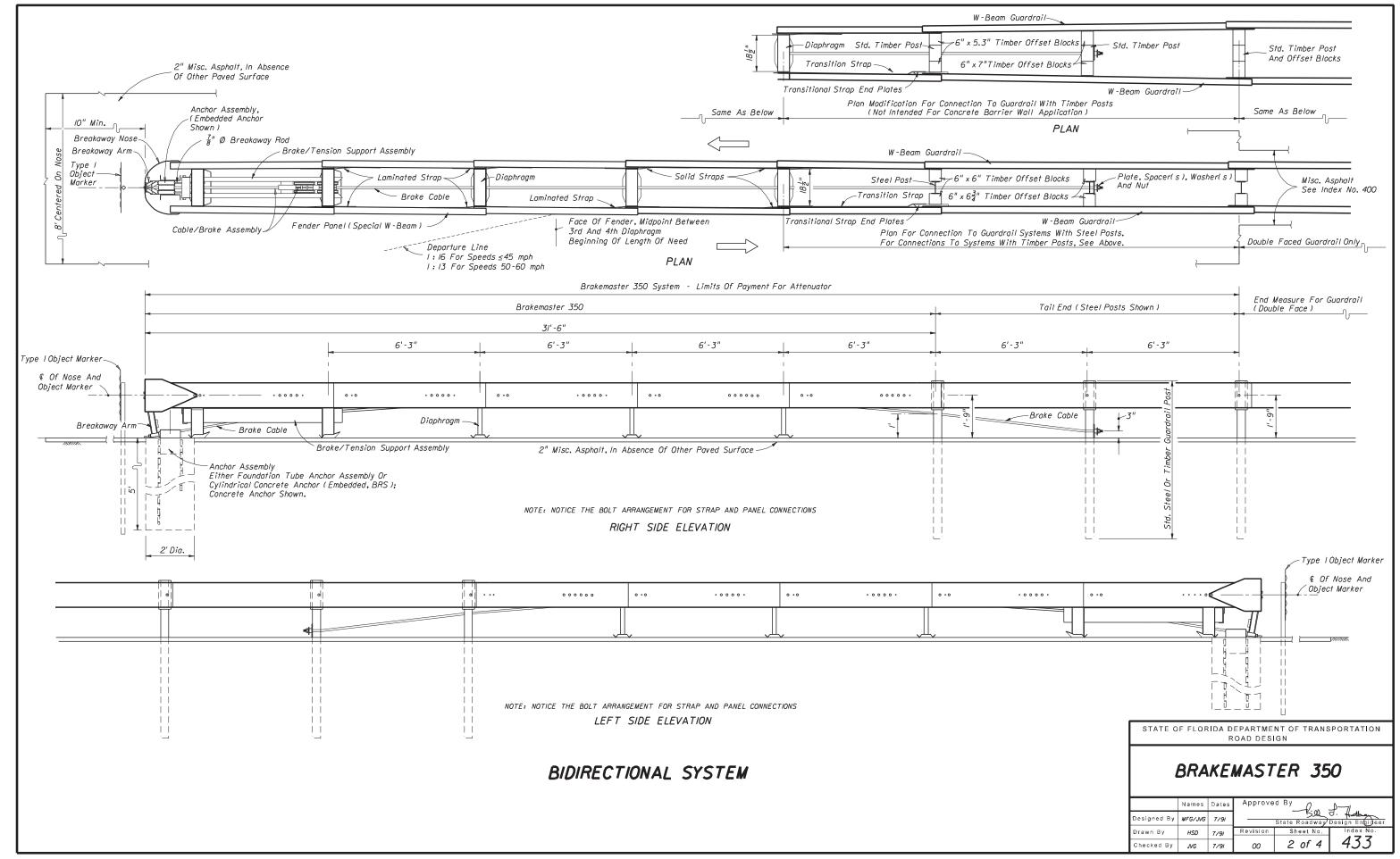
DESIGN NOTES AND GUIDELINES

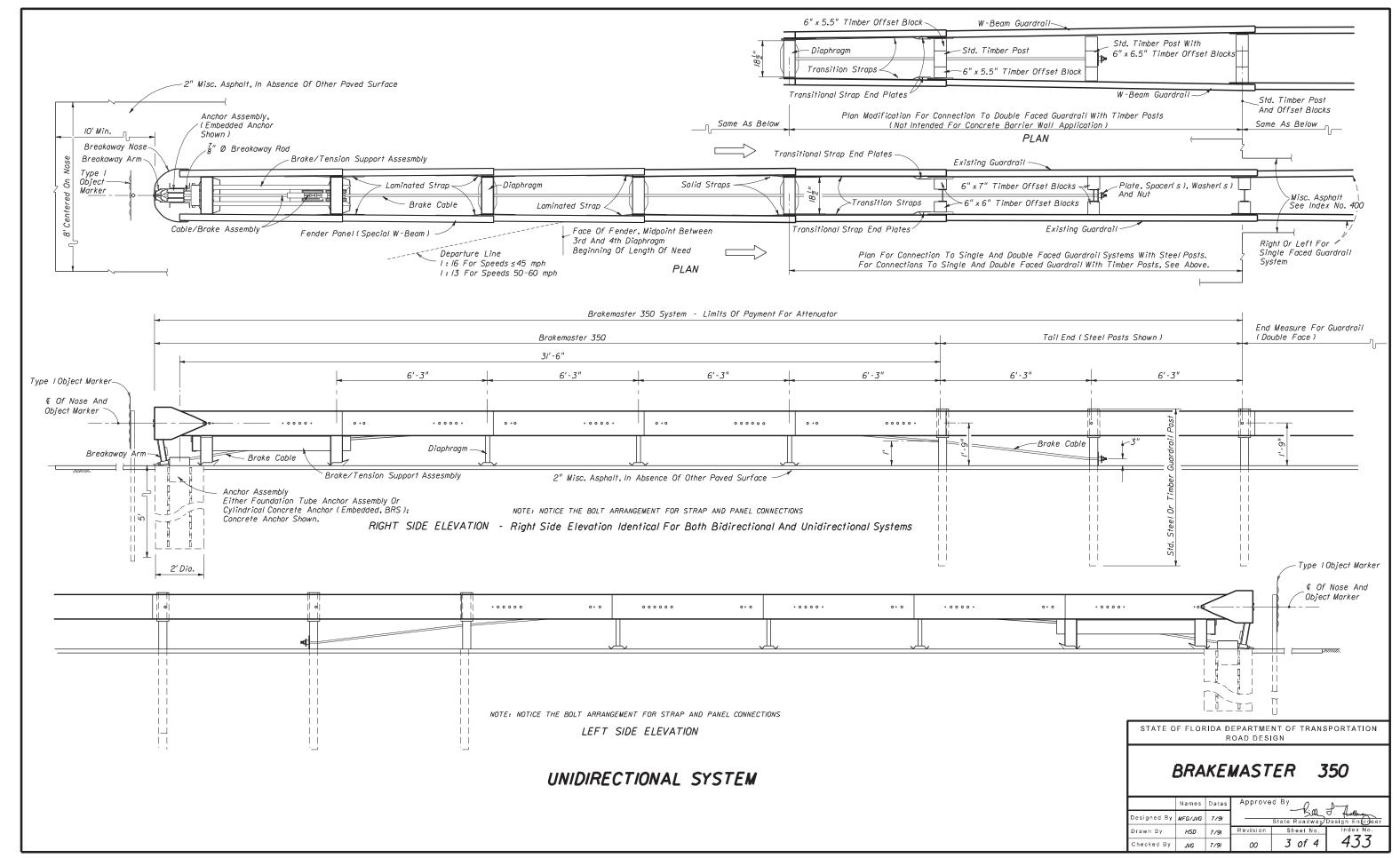
- I. The Brakemaster 350 system is designed to cushion automobile end-on hits and to redirect automobiles from side hits when impacting at speeds up to and including 60 mph. The Brakemaster 350 system has a singular design for all speeds of 60 mph or less, and any adjustment to its design will not be permitted except as authorized by the manufacturer.
- 2. The Brakemaster 350 system is specially designed to shield both narrow hazards and the ends of other fixed barriers located in low frequency impact areas. The Brakemaster 350 system is not intended for use in gores of freeways and expressway mainline ramp terminals; gores of roadway forks; and, other gore locations where there is a history of high frequency vehicle departures from the roadway or the potential exists for such departures. The Brakemaster 350 system is not a restorable design and therefore requires complete replacement after having sustained either an end-on or a side vehicular impact. Deformed side rail elements of the Brakemaster 350 will be subfunctional and are to be replaced immediately; deformed elements are not to be refurbished for reuse. When replacing an impacted Brakemaster 350 system the cable/brake assembly is not to be reused, if the cable sleeve is exposed. After vehicle impact on the Brakemaster 350 system the cable/brake assembly can be returned to the manufacturer for credit toward replacement of the cable.
- 3. Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the Brakemaster 350, and until such alternatives are available, the Brakemaster 350 need not be bid against other proprietary items.

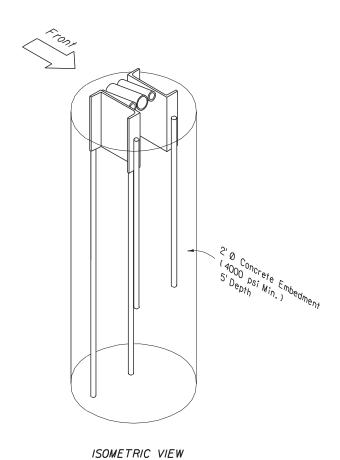
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

BRAKEMASTER 350

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	Designed By	MFG/JVG	7/91	State Roadway Design Engineer				
	Drawn By	HSD	7/9/	Revision	Sheet No.	Index No.		
	Checked By	JVG	7/91	00	1 of 4	433		

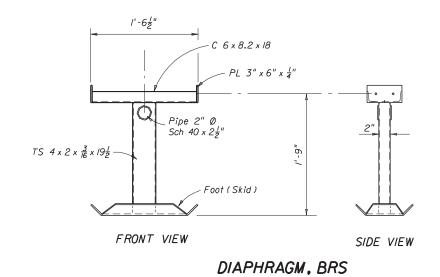


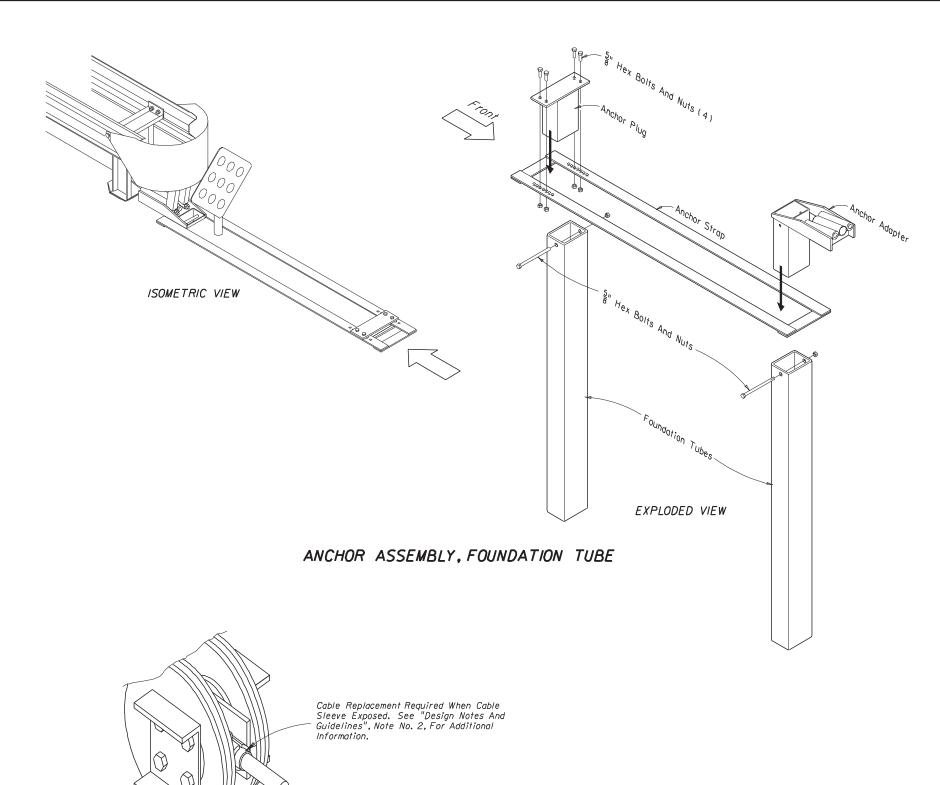




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ANCHOR ASSEMBLY, EMBEDDED BRS



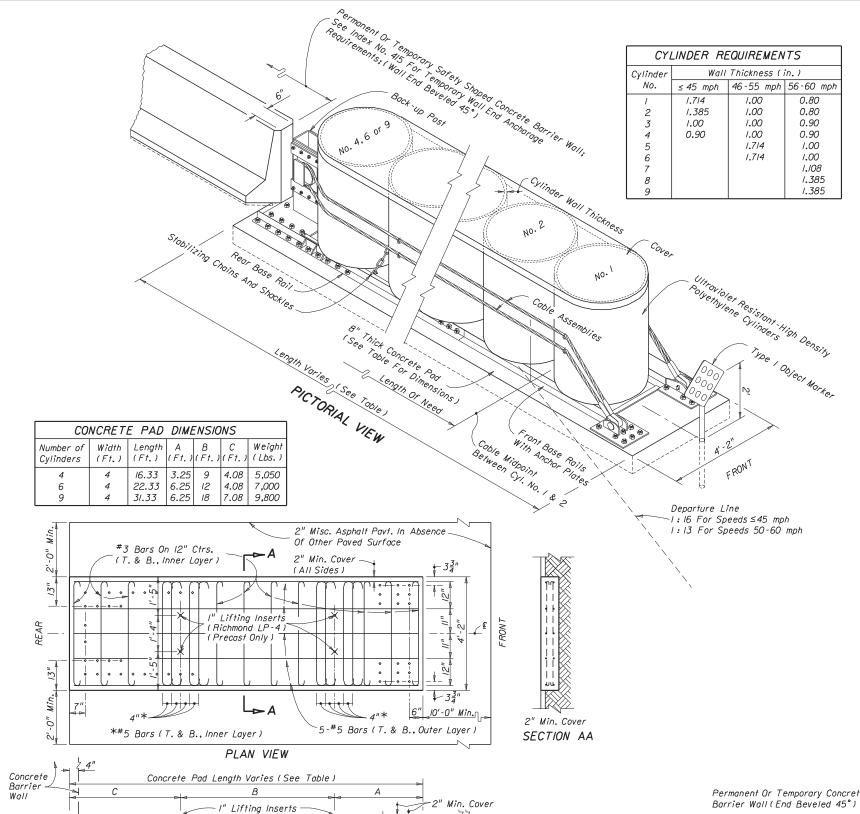


BRAKE/CABLE REPLACEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

BRAKEMASTER 350

	Names	Dates	Approve	4)	D-71
Designed By	MFG/JVG	7/91			Design Engineer
Drawn By	HSD	7/91	Revision	Sheet No.	Index No.
Checked By	JVG	7/91	00	4 of 4	433



cannot be relocated and do not require reinforcement.

ELEVATION

Note: Concrete pads may be precast or cast in place. Precast pads may be permanent or temporary and

CONCRETE PAD

can be relocated and require reinforcement. Cast in place pads can be permanent or temporary and

3" Min. C To C

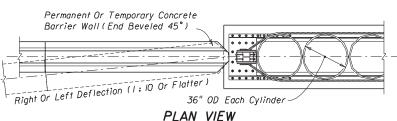
GENERAL NOTES

- I. The energy absorbing unit represented on this standard drawing is a proprietary design by Roadway Safety Service, Inc. and marketed under the trade name REACT 350, short for Reusable Energy Absorbing Crash Terminal. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for use by the Department and its assignees.

 This standard drawing provides the general graphics and information necessary to field identify component parts of the REACT 350 and their incorporation into a whole unit.
- 3. This standard drawing is sufficient for plan details for the REACT 350 installed as a free standing unit shielding safety shaped concrete barrier wall ends and for that use precludes the requirement for shop drawing submittals unless the plans otherwise call for such submittals. Use of the REACT 350 for shielding other hazards will require plan details, shop drawings, or both where called for in the plans.
- 4. The REACT 350 shall be assembled and installed in accordance with the manufacturer's detailed drawings, procedures and specifications.
- 5. Concrete foundations shall be constructed with 4000 psi min. compressive strength concrete.
- 6. The REACT 350 is suitable for speeds ≤60 mph.
- 7. The REACT 350 shall be constructed on cross slopes I: IO or flatter.
- 8. On facilities with speeds of ≤45 mph, the REACT 350 can be used in any location specified by the plans or by Department permit. On facilities with speeds of 50-60 mph, units shall not be used in narrow medians where post impact trajectory from end on crashes (rebound) will result in the crash vehicle rebounding into opposing traffic lanes, nor used in gore locations where the crash vehicle is likely to rebound into either the continuing or departing traffic lanes; units can be used in medians and gores where other features such as profile differentials, berms, ditches or other barriers will prevent adverse rebounding encroachment into traffic lanes.
- 9. Due to the overall unit height of 4'-0", which exceeds the drivers height of eye, caution is to be exercised in locating the REACT 350 to avoid blockage of required sight distance.
- IO. All metallic components shall meet the galvanizing requirements for quardrail, Index No. 400.
- II. A yellow Type I Object Marker shall be centered 3' in front of the nose of the REACT 350. Mounting hardware shall be in conformance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost of the REACT 350.
- 12. For REACT 350 units that have been impacted by vehicle crashes and are to remain in service, close inspection must be made on the anchorages of the front cable anchor plates and the rear pylon; the anchorages must be in design condition when restoration is complete.
- 13. Quantity for payment of both permanently and temporarily installed REACT 350 units will be based on each independent installation as called for in the plans or as directed by the Engineer. Payment for the permanently installed REACT 350 is for an assembled and installed system including the foundation, and will be paid for under the contract unit price for Impact Attenuator Vehicular (REACT 350), EA. Payment for the temporary REACT 350 is for an assembled and installed unit with components as described for the permanent installation with the addition of miscellaneous asphalt pavement and will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (REACT 350), LO, or when the REACT 350 is used as an option in accordance with Index No. 415 it will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

DESIGN NOTES

- I. The REACT 350 is designed to cushion automobile end-on hits and to redirect automobiles from side hits. The number of cylinders to be used in a specific unit will be determined by the design speed, except where the Engineer determines that another speed is more applicable.
- 2. The REACT 350 is a restorable system that is particularly suited to shielding hazards in areas with a history of frequent errant vehicle departures from the roadway or the potential exists for such departures. Until further development is completed in the application of the REACT 350 to shielding other hazards, this Index is limited to use with safety shaped concrete barrier walls. The REACT 350 alone is not suited to shielding a wide hazard.
- 3. The REACT 350 crash data accepted by the Federal Highway Administration (FHWA) covers vehicular impacts at speeds of 60 mph with 9 cylinder units and 45 mph with 4 cylinder units. The 6 cylinder unit has been developed by analytical deduction based on relative energy imparted by an impacting vehicle at various speeds. Until crash test data, accident data or other in service data is available to indicate change in application, the Department will support appropriate use of the six 6 cylinder units at locations where speeds are 55 mph or less. See 'CYLINDER REQUIREMENTS' table above.
- 4. The REACT 350 is a proprietary device with distinct performance, vehicular response and restoration characteristics, unlike other redirective crash cushions. Currently the Department recognizes the devices selective features and does not recognize other proprietary devices as equal alternatives, and until such alternatives are available the REACT 350 need not be bid against other proprietary items.



RFACT 350

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

REACT 350

	Names	Dates	Approve	d By	♪¬/
Designed By	MFG	8-95		State Roadway	Design Engineer
Drawn By	НКН	8 - 95	Revision	Sheet No.	Index No.
Checked By	JVG	8 - 95	00	I of I	434

QuadGuard SYSTEM 5'-9" / 7'-6" FOR WIDE HAZARDS QuadGuard SYSTEM 2'-0" / 2'-6" / 3'-0" FOR NARROW HAZARDS

GENERAL NOTES

- I. The energy absorbing system represented on this standard drawing is a proprietary design by Energy Absorption Systems, Inc. and marketed under the trade name QuadGuard. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for use by the Department and its assignees. This standard drawing provides the general graphics and information necessary to field identify component parts of the QuadGuard System and their incorporation into a whole system.
- 3. This standard drawing is sufficient for plan details for the QuadGuard installed as a free standing system or installed in connection with concrete barrier walls and other fixed barrier systems, and precludes the requirement for shop drawing submittals unless the plans otherwise call for such submittals.

The QuadGuard tension strut backup is the primary backup to be used on Florida Department Of Transportation projects. Use of concrete backups will be permitted, but will require call out and detailing in the plans for site specific construction; concrete backups must meet manufacturers specifications, installation guidelines and transition hardware requirements.

- 4. The QuadGuard shall be assembled and installed in accordance with the manufacturers detailed drawings, procedures and specifications.
- 5. The QuadGuard is available in 24", 30", and 36" nominal widths for narrow hazards and 69" and 90" nominal widths for wide hazards. The system width will be as called out in the plans, permit or other contract document for each location.
- 6. Only the QuadGuard Type I and Type II cartridges shall be used in bay and nose locations as described in the 'BAY SELECTION GUIDELINES' table.
- 7. Cement concrete foundations and cement concrete backup assemblies shall be constructed with 4000 psi min. compressive strength concrete.
- 8. The QuadGuard shall be constructed on cross slopes I: 10 or flatter.
- 9. All metallic components shall meet the galvanizing requirements for guardrail, Index No. 400.
- IO. A yellow Type I Object Marker shall be centered 3' in front of the nose of the QuadGuard. Mounting hardware shall be in conformance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost of the QuadGuard.
- II. Quantity for payment is based on each independent location as called for in the plans or as directed by the Engineer. The cost for foundations, subgrade preparation and miscellaneous asphalt shown on this index will be included in the cost for the QuadGuard system. The permanent QuadGuard System will be paid for under the contract unit price for Impact Attenuator Vehicular (QuadGuard), EA; temporary units will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (QuadGuard), LO, or when the QuadGuard system is used as an option in accordance with Index No. 415, it will be paid for under contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

DESIGN NOTES AND GUIDELINES

- I. The QuadGuard System is designed to cushion automobile end-on hits and to redirect automobiles from side hits. The QuadGuard is designed to shield fixed hazards or the ends of other temporary and permanent barrier systems. The number of bays to be used in a specific unit will be determined by the design speed, except where the Engineer determines that another speed is more applicable. The unit width will be determined by the width of the object to be shielded or by the connecting barrier system. The backup assembly for a specific unit will be determined by either (a) the unit standing free of the object to be shielded or (b) the barrier system(s) to which it is connected.
- 2. The QuadGuard is a restorable system that is particularly suited to shielding hazards subject to high speed traffic, high volume traffic, and/or traffic with a history of frequent errant vehicle departures from the roadway or the potential exists for such departures. The QuadGuard is particularly suited to shielding hazards where the approach space is limited; and, is particularly suited to conditions where the terminal must be located close to the traffic lane.
- 3. Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the QuadGuard, and until such alternatives are available, the QuadGuard need not be bid against other proprietary items. However, for temporary use where the QuadGuard and other approved redirective crash cushions meet or exceed the minimum requirements for a specific location, the approved crash cushions will be considered optional systems and paid for as described in General Note II above.

BAY SELECTION GUIDELINES						
Speed	No. Of	Number Of Cartridges		Length		
(mph)	Bays	Type I (Front)	Type II (Rear)	,		
≤ 40	2	2	1	8'-8"		
45	3	3	1	//'-8"		
50	4	3	2	14' -8"		
55	5	4	2	17'-8"		
60	6	4	3	20'-8"		
65	7	4	4	23'-8"		
70	9	4	6	29' -8"		

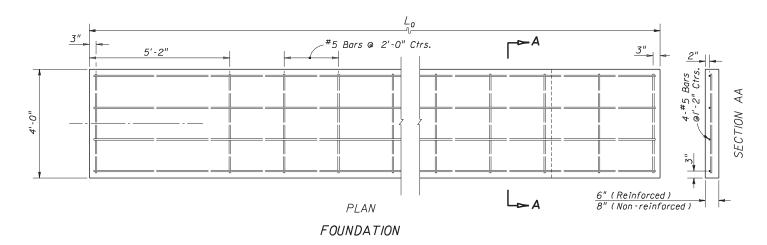
The Manufacturer provides QuadGuard units with up to 12 bays designed for use with speeds up to 75 mph. These larger units may be utilized when called for in the plans or as directed by the Engineer.

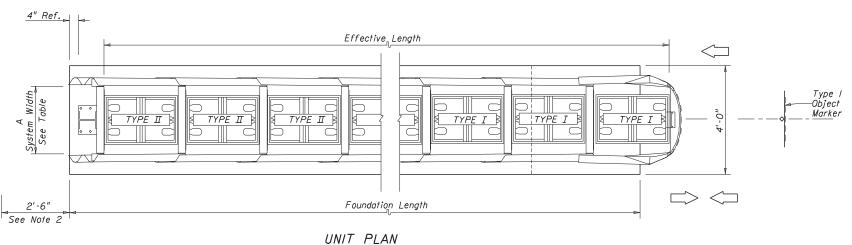
GENERAL SYSTEM FEATURES AND BAY SELECTION GUIDELINES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

QuadGuard

	Names	Dates	Approve	d By	P-71
Designed By	MFG/STD			State Roadway	Design Engineer
Drawn By	HKH		Revision	Sheet No.	Index No.
Checked By	JVG		00	1 of 6	435

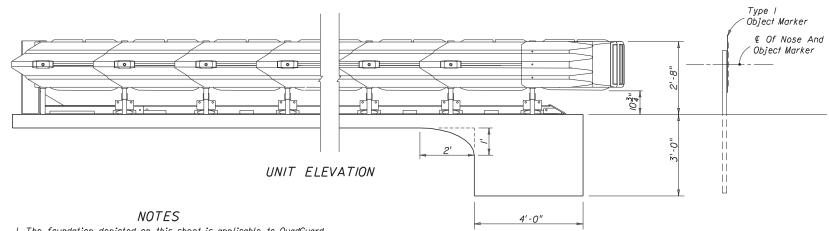




Nominal	Α		
System Width	(Backup Width)		
2'-0"	2'-0"		
2'-6"	2'-6"		
3'-0"	3'-0"		
5'-9"	5' - 3 3 "		
7'-6"	6' - 10 5 "		

	ESTIMATED FOUNDATION QUANTITIES								
	For Informational Purposes Only								
N= 06	REINFORCED NON - REINFORCED								
No. Of Bays	Lo	Rebar Required	Concrete Required	Concrete Required					
		110401100	(CY)	(CY)					
2	9'-0"	48'-8"	2.1	2.3					
3	12'-0"	68'-0"	2.4	2.7					
4	/5'-0"	83'-8"	2.6	3.0					
5	18'-0"	103'-0"	2.8	3.3					
6	21'-0"	118' -8"	3./	3.6					
7	24'-0"	138'-0"	3.3	3.9					
9	30' -0"	173'-0"	3.7	4.5					

Note: Monorail anchorage bolt spacing to be in accordance with the manufacturers installation drawings and specifications.



- I. The foundation depicted on this sheet is applicable to QuadGuard systems for both narrow and wide hazards, 2'-6" system shown.
- 2. For the number of bays required see table, Sheet I.
- 3. Provision shall be made for rear fender panels to slide rearward upon impact 2'-6" min.
- 4. For barrier connections see 'TRANSITIONS', Sheet Nos. 4 and 5.

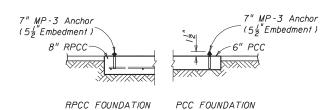
PERMANENT FOUNDATION FOR TENSION STRUT BACKUP ASSEMBLY

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

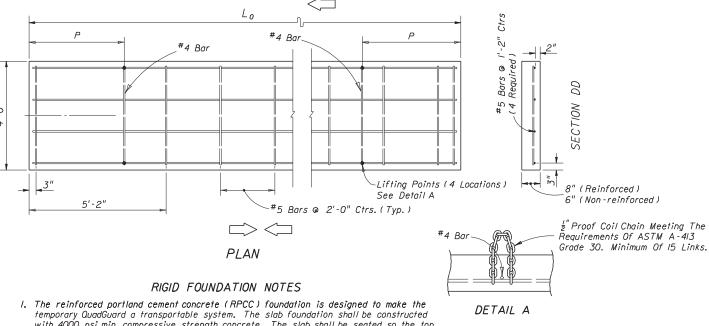
QuadGuard

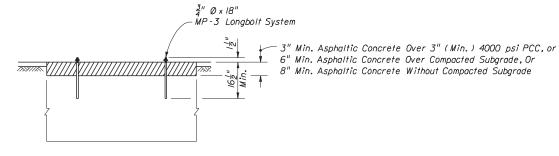
	Names	Dates	Approve	d By	
Designed By	MFG/STD			State Roadway	Design Engineer
Drawn By	HKH		Revision	Sheet No.	Index No.
Checked By	JVG		00	2 of 6	435

ESTIMATED FOUNDATION QUANTITIES For Informational Purposes Only								
		_	REINFORCED		NON - REINFORCED			
	No. Of Bays	P	Rebar	Concrete	Concrete			
Вауз			Required	Required (CY)	Required (CY)			
3	12'	3'-0"	68'-0"	1.2	0.9			
4	15'	3'-9"	83'-8"	1.5	1.1			
5	18'	4'-6"	103'-0"	1.8	1.3			
6	21'	5'-3"	118' -8"	2.1	1.6			
7	24'	6'-0"	/38'-0"	2.4	1.8			
9	30'	7'-6"	173'-0"	3.0	2.2			

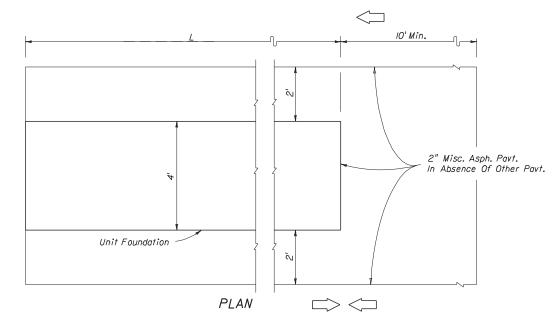


PARTIAL SECTIONS





SECTION



MP-3 LONGBOLT ANCHOR SYSTEM

ASPHALTIC CONCRETE FOUNDATIONS

- 1. The reintorced portland cement concrete (RPCC) toundation is designed to make the temporary QuadGuard a transportable system. The slab foundation shall be constructed with 4000 psi min. compressive strength concrete. The slab shall be seated so the top of the slab is flush with the surface intended for approaching vehicles. In absence of other pavement the surrounding surface shall be paved with 2" of miscellaneous asphalt pavement as depicted in 'ASPHALTIC CONCRETE FOUNDATIONS'. The QuadGuard shall be anchored exclusively with the 7" MP-3 anchor system supplied with the QuadGuard unit, unless another anchor is supplied or approved by the QuadGuard manufacturer.
- 2. The nonreinforced portland cement concrete (PCC) foundation shall be Class I concrete, having depth equal to or greater than 6". The PCC foundation utilization options are as follows: (a) Poured in place as an expendable slab, having a thickness of not less than 6"; disposal of the slab will be as approved by the Engineer, (b) Project constructed roadway PCC pavement, or, (c) Existing 9" PCC roadway pavement.

The utilization option applied shall be as approved by the Engineer on a site specific basis. The top of the foundation shall be flush with the surface intended for approaching vehicles. In absence of surrounding pavement the surrounding surface shall be paved as shown on this sheet in 'ASPHALTIC CONCRETE FOUNDATIONS'.

The QuadGuard installed on PCC pavement shall be anchored only with the MP-3 anchor system supplied with the QuadGuard unit. Holes for the 7" anchors shall be drilled in both existing and new pavements. When the QuadGuard is removed from the project pavement or from existing pavement that is to remain in place, the anchor shall be cut off flush with the top of the pavement, unless the plans call for other treatment.

3. For additional information see the General Notes.

REINFORCED AND NONREINFORCED CONCRETE PAD SYSTEMS

CEMENT CONCRETE FOUNDATIONS

NOTES

- I. For the number of bays required see table, Sheet I.
- 2. For barrier connections see 'TRANSITIONS', Sheet Nos. 4 and 5.

TEMPORARY FOUNDATIONS

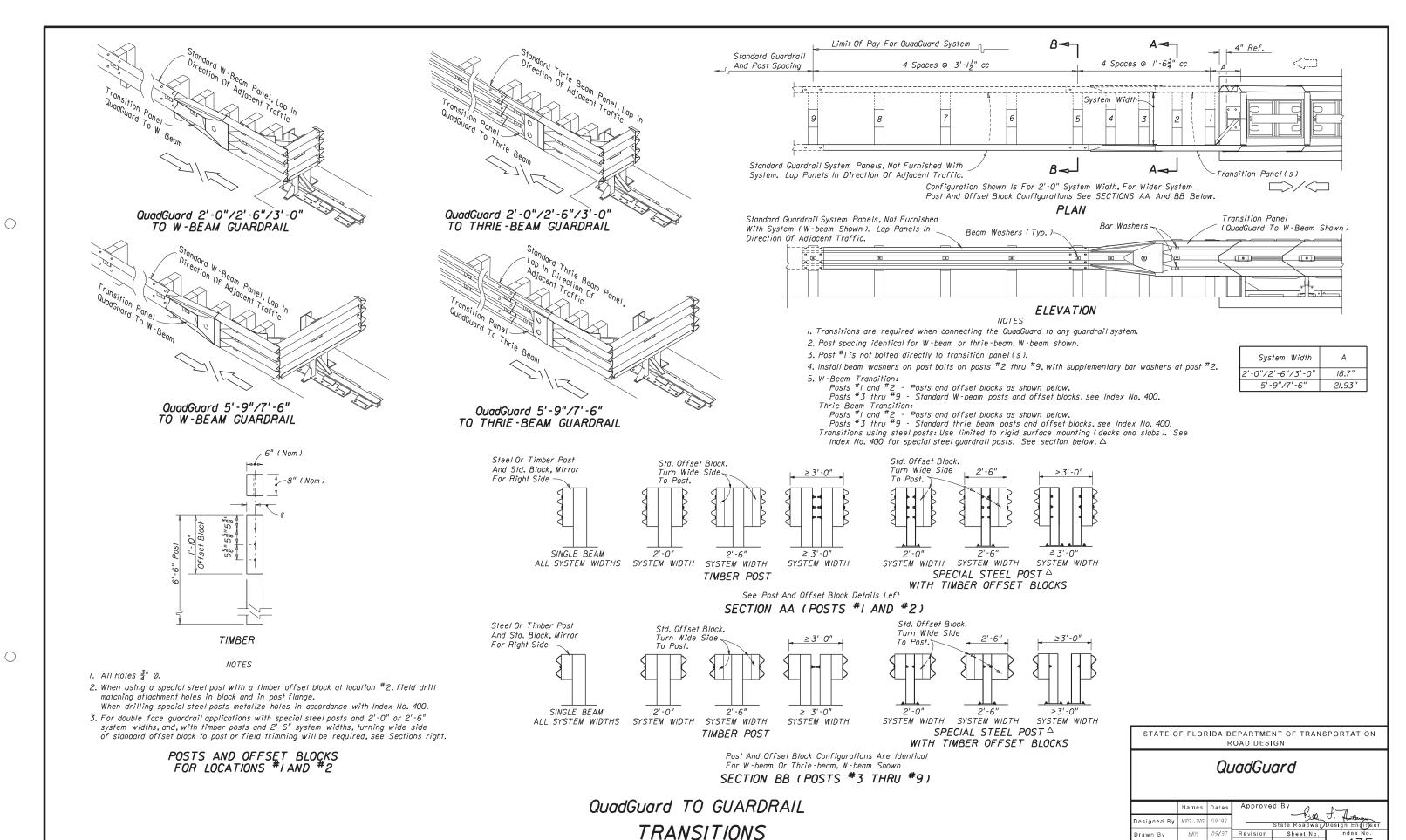
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

QuadGuard

Names Dates Approved By

Designed By MFG State Roadway Design Engineer

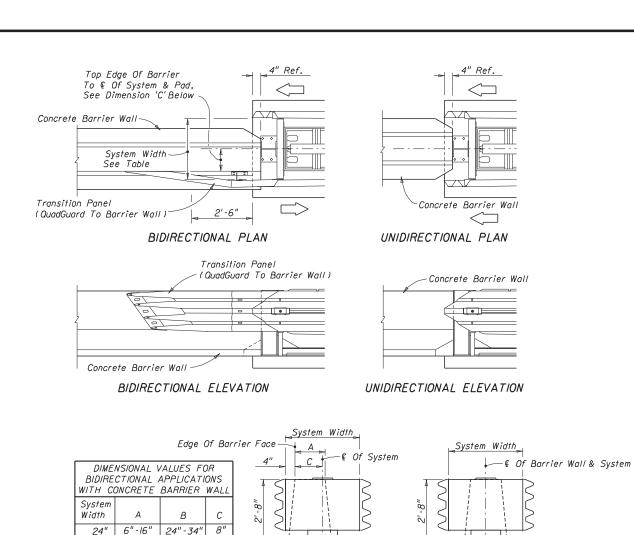
Drawn By HKH 8/97 Revision Sheet No. Index No. Checked By JVG 8/97 00 3 of 6 435



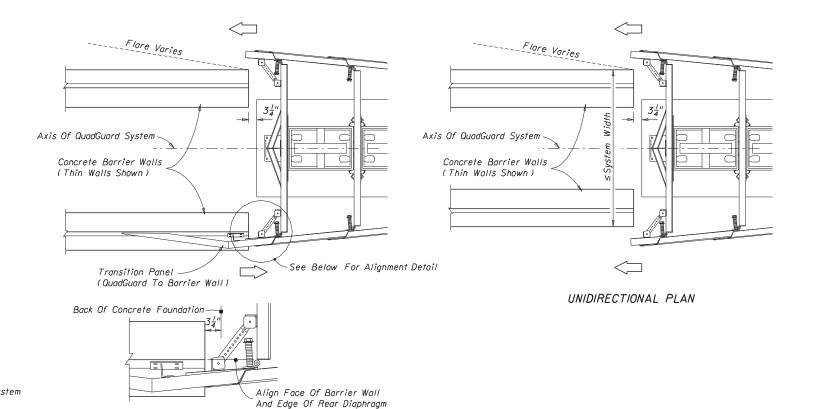
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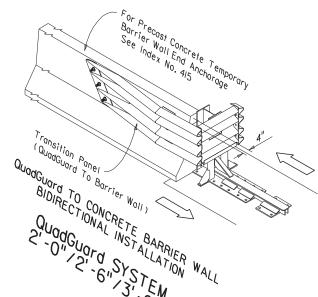


ALIGNMENT DETAIL

BIDIRECTIONAL PLAN

The axis of the QuadGuard relative to concrete barriers will be established on site specific basis. The QuadGuard supplier shall furnish dimensional data for setback between the barrier wall end and the system foundation, and for the alignment between the face of the barrier wall and the rear diaphragm where dimensions other than those above apply.

QuadGuard SYSTEM 5'-9" / 7'-6"



BIDIRECTIONAL SECTION

30" | 12" - 22" | 30" - 40"

36" | 18" - 28" | 36" - 46" | 14"

UNIDIRECTIONAL SECTION

Barrier Wall Width

[¯]≤System Width

BARRIER WALL TRANSITION NOTE

Barrier wall free end must be reinforced in accordance with Index No. 4/0 and temporary walls must be adequately anchored for proper impact performance in accordance with Index No. 4/5.

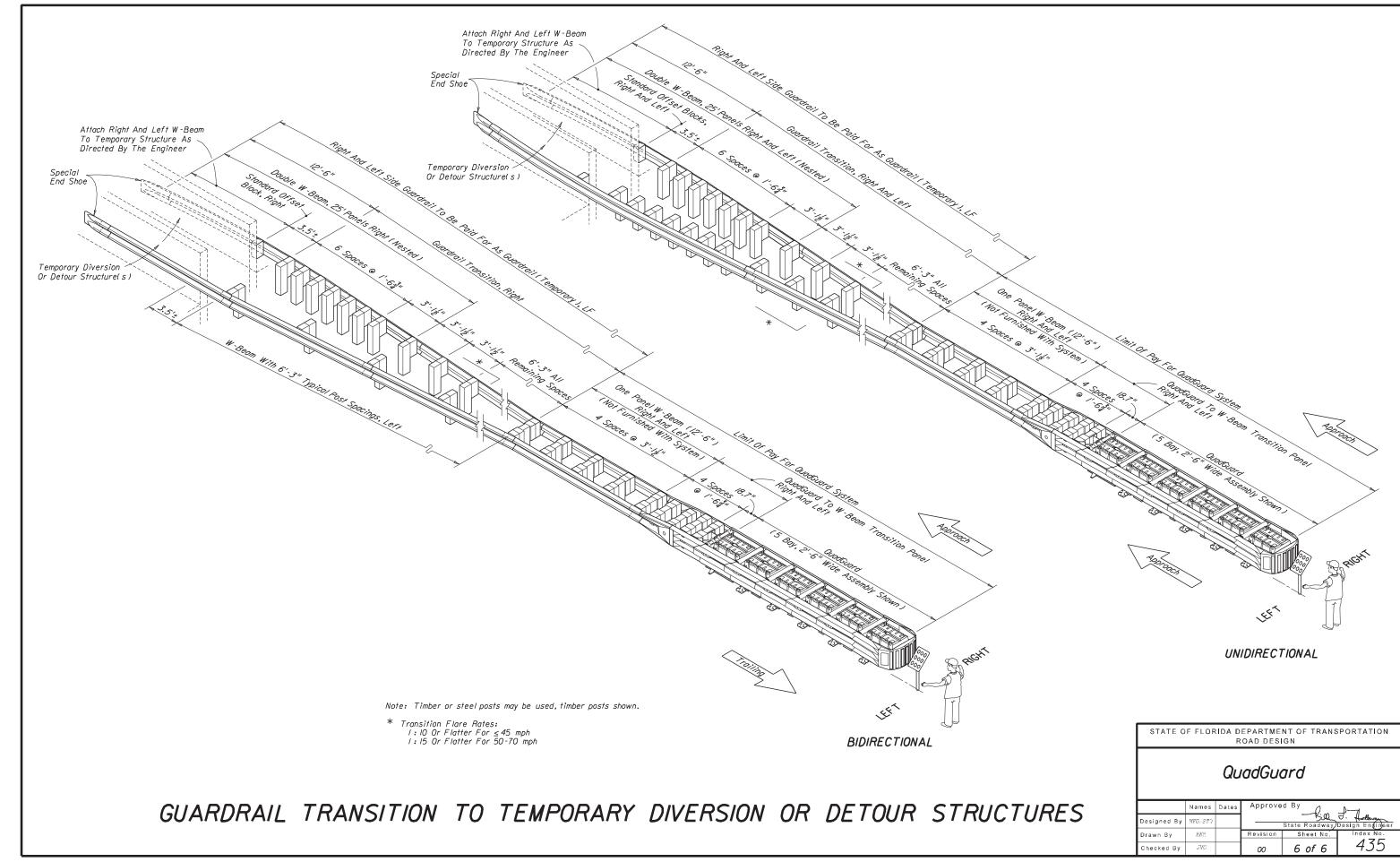
QuadGuard TO CONCRETE BARRIER WALL

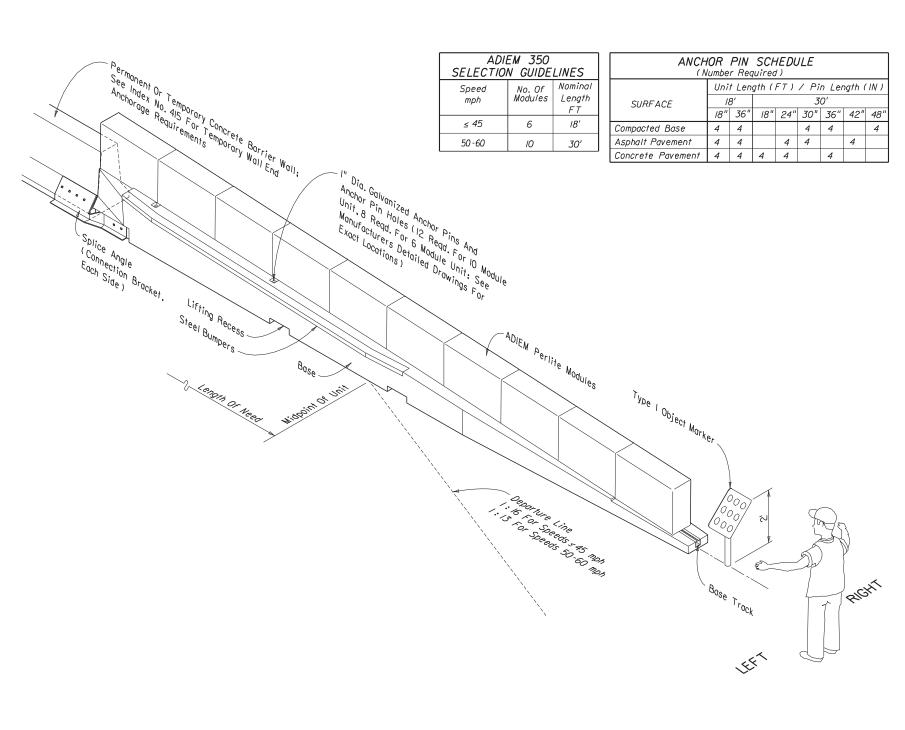
TRANSITIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

QuadGuard

	Names	Dates	Approve	d By	P-71
Designed By	MFG/JVG	09/97		State Roadway	Design Engineer
Drawn By	HKH	09/97	Revision	Sheet No.	Index No.
Checked By	JVG	09/97	00	5 of 6	435





GENERAL NOTES

- I. The energy absorbing system represented on this standard drawing is a proprietary design by SYRO Inc. and marketed under the trade name ADIEM 350. Any infringement on the rights of the designer shall be the sole responsibility of the user.
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for use by the Department and its assignees. This standard drawing provides the general graphics and information necessary to field identify component parts of the ADIEM 350 and their incorporation into a whole system.
- 3. This standard drawing is sufficient for plan details for the ADIEM 350 installed in connection with permanent or temporary concrete barrier walls, and precludes the requirement for shop drawing submittals unless the plans otherwise call for such submittals.
- 4. The ADIEM 350 shall be assembled and installed in accordance with the manufacturers detailed drawings, procedures and specifications.
- 5. The ADIEM 350 can be located on compacted base, asphalt or concrete. Driving of anchor pins into compacted base or soft asphalt will be permitted while drilling will be necessary for hard asphalt or concrete pavements. See schedule left for anchor pin requirements.
- 6. The ADIEM 350 is suitable for speeds ≤60 mph.
- 7. The ADIEM 350 shall be located parallel to the approach travel lane(s), on 1:10 or flatter cross slopes. Until there is further development in the application of the ADIEM 350, the system is not to be located in narrow medians, gores or locations where frequent side impacts can be expected.
- 8. All modules are alike in size and mass (interchangable).
- 9. Due to the overall unit height of 4', which exceeds the drivers height of eye, caution is to be exercised in locating the ADIEM 350 to avoid blockage of required sight distance.
- 10. Attach splice angle (connection bracket) to ADIEM 350 base with $2 \cdot l \frac{f}{g}$ dia. x 25" long HD hex bolts. Attach splice angle to barrier wall with 8 field drilled $\frac{7}{g}$ dia. x 6" long chemical anchors.
- II. A yellow Type I Object Marker shall be centered 3' in front of the nose of the ADIEM 350. Mounting hardware shall be in conformance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost of the ADIEM 350.
- 12. Temporary ADIEM 350 systems can be reused provided the bases have the structural integrity and surface qualities of new systems, and the modules are condition new. Refurbished systems can be made up of mixed new and used components. New and used systems can be purchased, leased, rented, on loan or shared between projects.
- 13. The permanent ADIEM 350 will be paid for under the contract unit price for Impact Attenuator Vehicular (ADIEM), EA; temporary units will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (ADIEM), LO, or when the ADIEM 350 is used as an option in accordance with Index No. 415, it will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

DESIGN AND MAINTENANCE NOTES AND GUIDELINES

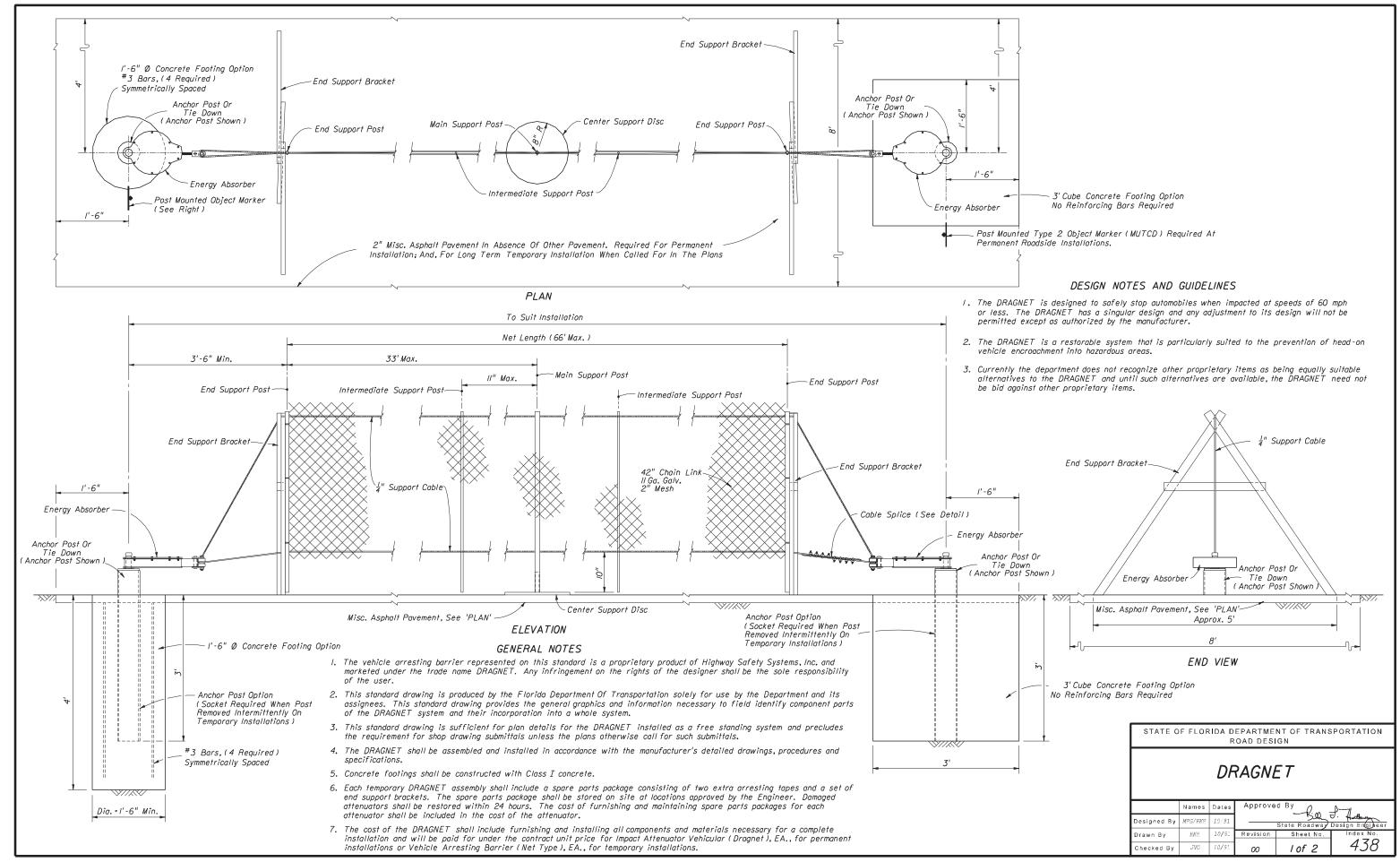
- I. The ADIEM 350 is designed to cushion automobile end-on hits and to redirect automobiles from side hits within the length of need while shielding the ends of permanent or temporary concrete barrier walls.
- 2. The ADIEM 350 is a restorable system that is particularly suited to shielding concrete barrier wall ends. The I8' unit is applicable for speeds of 45 mph or less, the 30' unit is applicable for speeds of 50-60 mph.
- 3. The upstream half of the system (3 or 5 modules) is a gating design. Each module (cartridge) has a mass of 180 lbs. Care must be exercised in locating the system where debris scatter may pose a hazard. Upstream modules or their residual components must be removed to replace damaged downstream modules.
- 4. The ADIEM 350 will require close monitoring for damage that will open module encasement; immediate repair is essential to prevent moisture absorption into module core.
- 5. Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the ADIEM 350, and until such alternatives are available, the ADIEM 350 need not be bid against other proprietary items. However, where the ADIEM 350 and other approved temporary redirective crash cushions meet or exceed the minimum requirements for a specific location, the approved crash cushions will be considered optional systems and paid for as described in General Note 13 above.

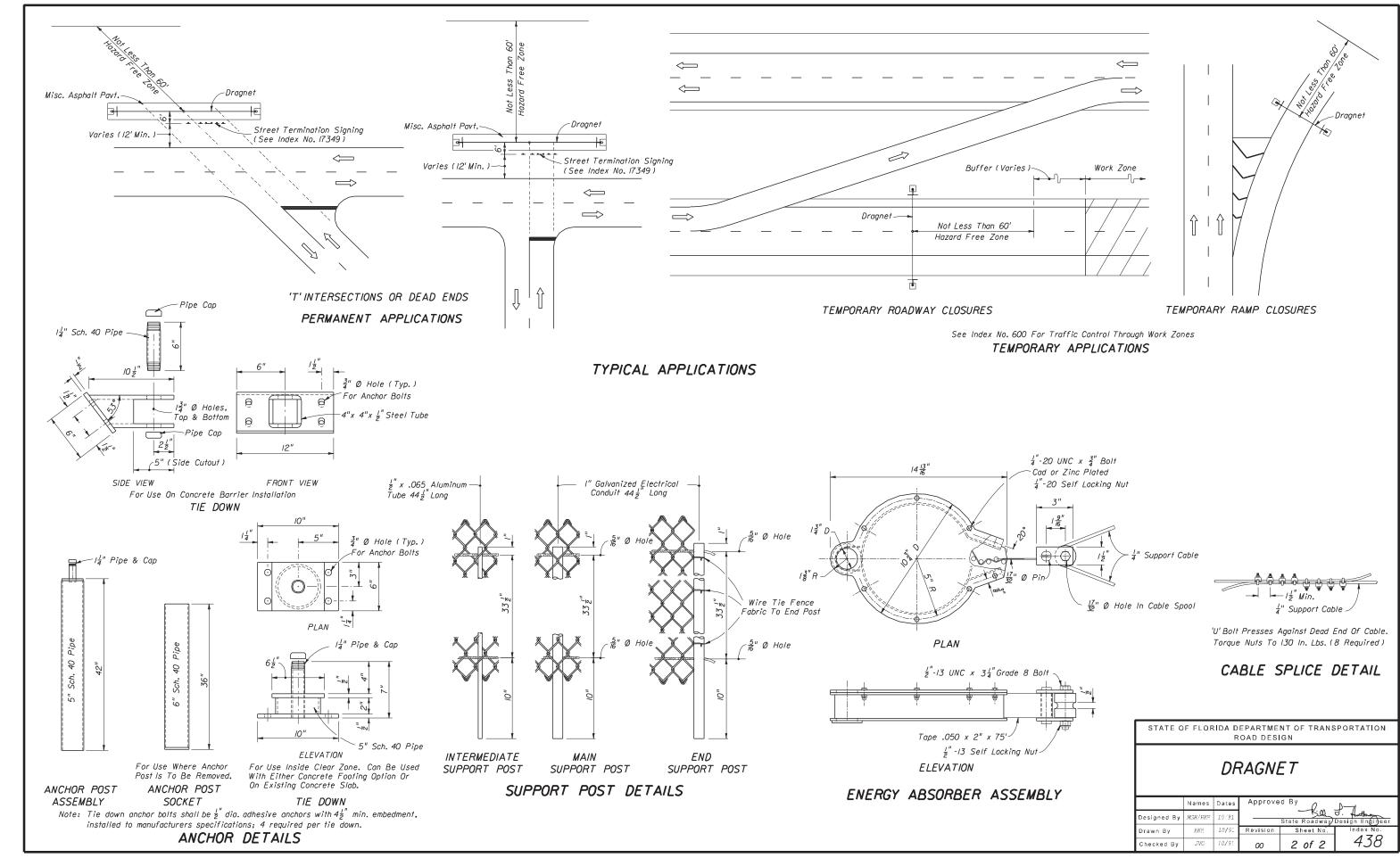
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

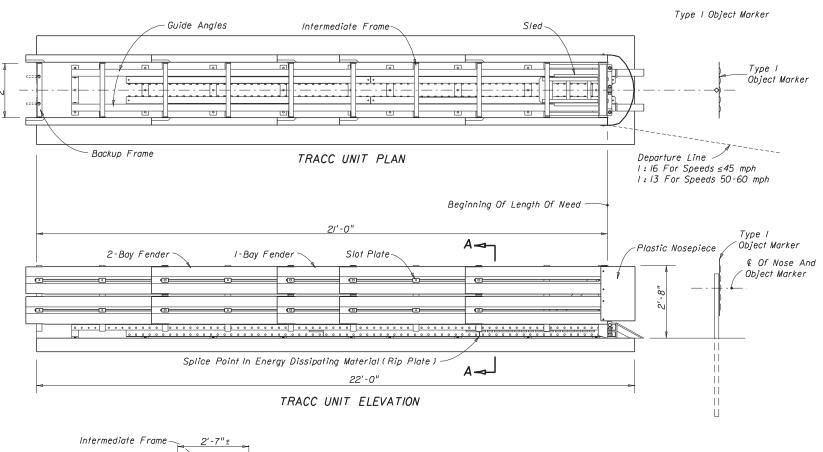
ADIEM 350

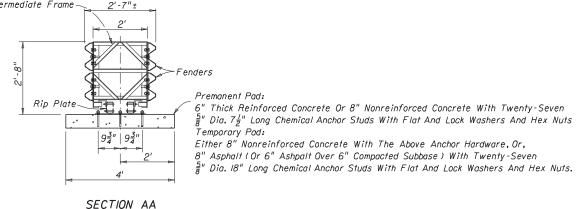
	Names	Dates	Approve	d By	1
Designed By	MFG			State Roadway	Design Engineer
Drawn By	HKH	7/97	Revision	Sheet No.	Index No.
Checked By	JVG	7/97	00	I of I	436

GENERAL SYSTEM FEATURES AND GUIDELINES









GENERAL NOTES

- I. The energy absorbing system represented on this standard drawing is a proprietary design by Trinity Industries, Inc. and marketed under the trade name TRACC. Any infringement on the rights of the designer shall be the sole responsibility of the user
- 2. This standard drawing is produced by the Florida Department Of Transportation solely for use by the Department and its assignees. This standard drawing provides the general graphics and information necessary to field identify component parts of the TRACC and their incorporation into a whole system.
- 3. At time of publishing this standard the TRACC system is limited to installations on bulbed-out ends of concrete barrier walls. Under this application the standard drawing is sufficient for plan details, and precludes the requirement for shop drawing submittals unless the plans call for such submittals. See DESIGN NOTES for additional information.
- 4. The TRACC shall be assembled and installed in accordance with the manufacturers detailed drawings, procedures and specifications, except that transition section posts will be set to connect to guardrail at standard W-beam center bolt height (I'-9").
- 5. The TRACC system is suitable for speeds of ≤60 mph.
- 6. When the TRACC is installed at permanent locations it shall be anchored to either a reinforced 6" thick concrete pad or a nonreinforced 8" thick concrete pad with twenty-seven $7\frac{1}{2}$ " long $\frac{5}{8}$ " dia. chemical anchor studs, flat and lock washers, and, hex nuts. When the TRACC is installed at temporary locations it shall be anchored to a nonreinforced 8" thick concrete pad with the above mentioned anchor hardware, or a 8" thick asphalt pad (or a 6" thick asphalt over 6" of compacted subbase) using twenty-seven I8" long $\frac{5}{8}$ " dia. Grade 5 threaded chemical anchor studs, flat and lock washers, and, hex nuts.
- 7. The TRACC shall be located parallel to the approach travel lane(s), on l: 10 or flatter cross slopes.
- 8. In-place repairs on the TRACC crash cushion are limited to (a) end-on impacts which cause the sled to stroke 54" or less, and (b) side impacts where permanent distortion is limited to a unit's fender panels and where distortion of the intermediate frame(s) can be restored manually. Unit replacement is required when damage exceeds these conditions. Temporary construction units and units under Maintenance responsibility may be shop repaired units utilizing new or salvaged parts which will produce condition new units. All permanent units shall be factory new at completion of construction.
- 9. A yellow Type I Object Marker shall be centered 3' in front of the nose of the TRACC. Mounting hardware shall be in conformance with Index Nos. II860 and II865. The cost of the Object Marker shall be included in the cost of the TRACC.
- 10. The permanent TRACC will be paid for under the contract unit price for Impact Attenuator Vehicular (TRACC), EA; temporary units will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (TRACC), LO, or when the TRACC is used as an option in accordance with Index No. 4/5, it will be paid for under the contract unit price for Vehicular Impact Attenuator (Temporary) (Redirective Option), LO.

DESIGN AND NOTES AND GUIDELINES

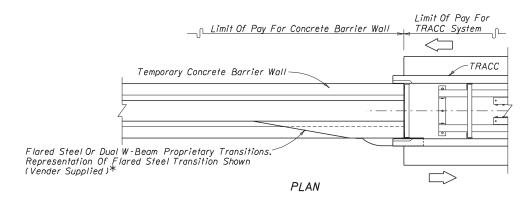
- I. The TRACC is designed to cushion automobile end-on hits and to redirect automobiles from side hits within the length of need while shielding the ends of permanent or temporary concrete barrier walls, or, double faced guardrails. At time of publishing this standard the manufacturer's designs for bidirectional hardware was complete for applications indicated on Sheet 2, but final acceptance and department approval had not been attained. For current status of development and acceptance contact the central Office of Roadway Design.
- 2. The TRACC system is not intended for use in gores of freeway and expressway mainline ramp terminals; gores of roadway forks; or other gore locations where there is a history of high frequency vehicle departure from the roadway or the potential exists for such departures. The TRACC system is not a restorable design, and repairs or replacement will be in accordance with GENERAL NOTES note No. 8.
- 3. Currently the Department does not recognize other proprietary items as being equally suitable alternatives to the TRACC, and until such alternatives are available, the TRACC need not be bid against other proprietary items. However, where the TRACC and other approved temporary redirective crash cushions meet or exceed the minimum requirements for a specific location, the approved crash cushions will be considered optional systems and paid for as described in General Note 10 above.

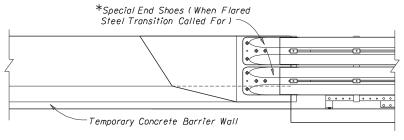
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRACC

| Names | Dates | Approved By | State Roadway/Design Engineer | State Roadway/Design Engineer | Drawn By | EKH. | 7/97 | Revision | Sheet No. | Index No. | Checked By | JVG | 7/97 | 00 | 1/0f 2 | 440

GENERAL SYSTEM FEATURES AND GUIDELINES

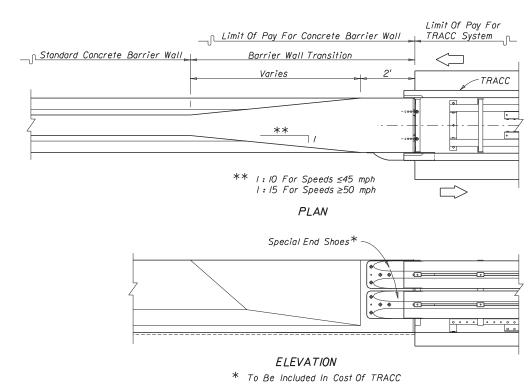




ELEVATION

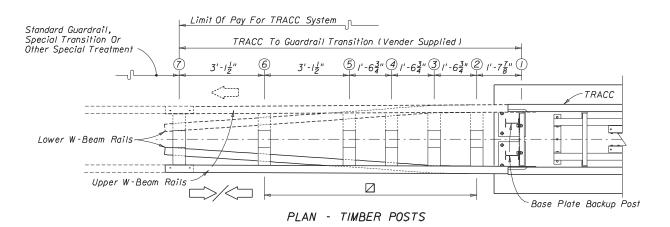
* To Be Included In Cost Of TRACC

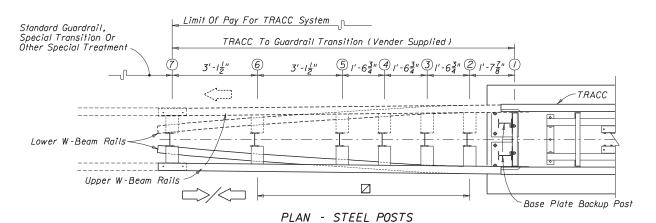
MANUFACTURED STEEL TRANSITION

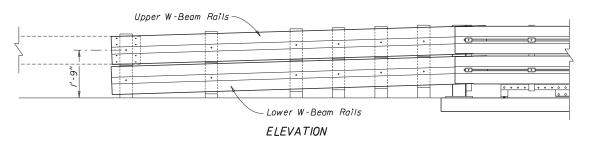


INTEGRAL WALL TRANSITION

TRACC TO CONCRETE BARRIER WALL





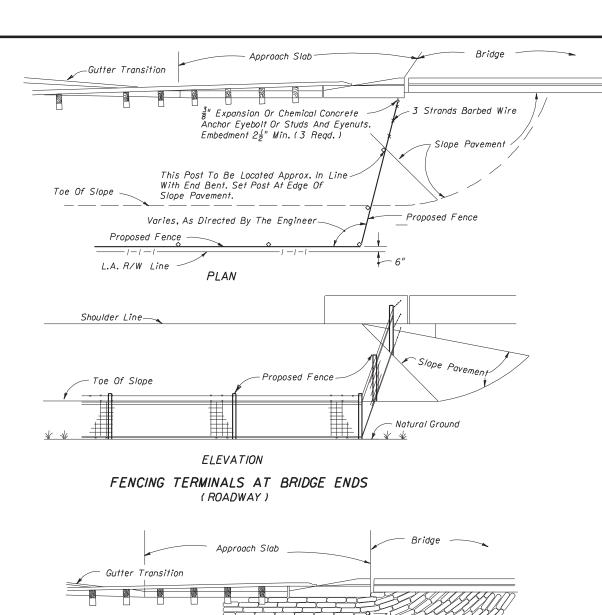


Offset blocks that exceed standard block depth can be made up of blocks of special size or multiple standard blocks field trimed to approximately equal size to achieve full transition width. Offset blocks for lower W-beam that are less in depth than standard blocks may be field trimed standard blocks. All blocks are to be secured to plan position by I6d galvanized nails.

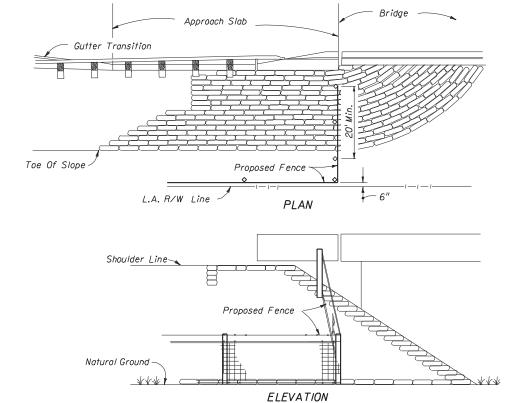
Transitions are required when connecting the TRACC to any guardrail system.

TRACC TO GUARDRAIL

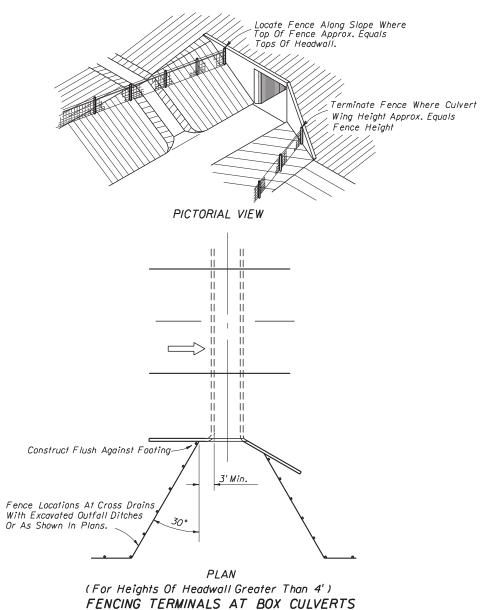
TRACC TRANSITIONS

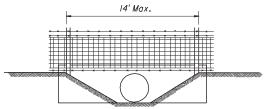


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FENCING TERMINALS AT BRIDGE ENDS
(STREAM CROSSING)





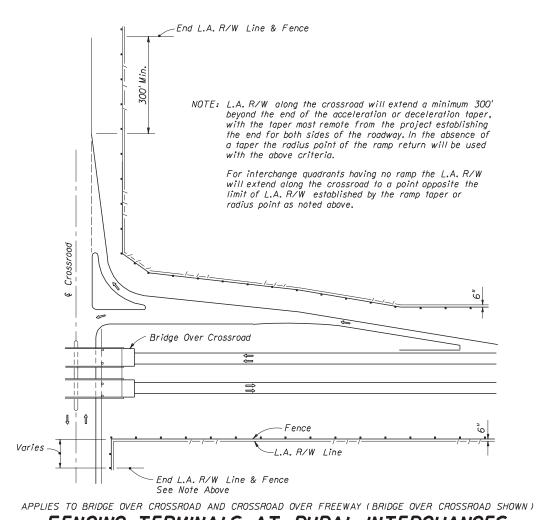
FENCING DETAIL AT CULVERT
(For Heights Of Headwalls 4' Or Less.)

Note: When height of headwall is 4' or less (drainage pipe 36" or less) the fence shall not be tied to the headwall, but shall span the lateral ditch.

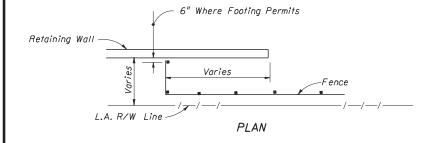
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

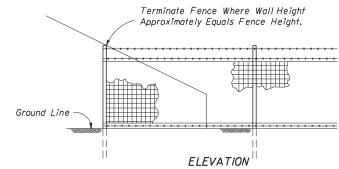
FENCE LOCATION

		Names	Dates	Approve	d By	♪ /
Des	igned By	HFW	02/65		State Roadway	Design Engineer
Dra	wn By	HFW	02/65	Revision	Sheet No.	Index No.
Che	cked By	RLO	02/65	00	1 of 2	450

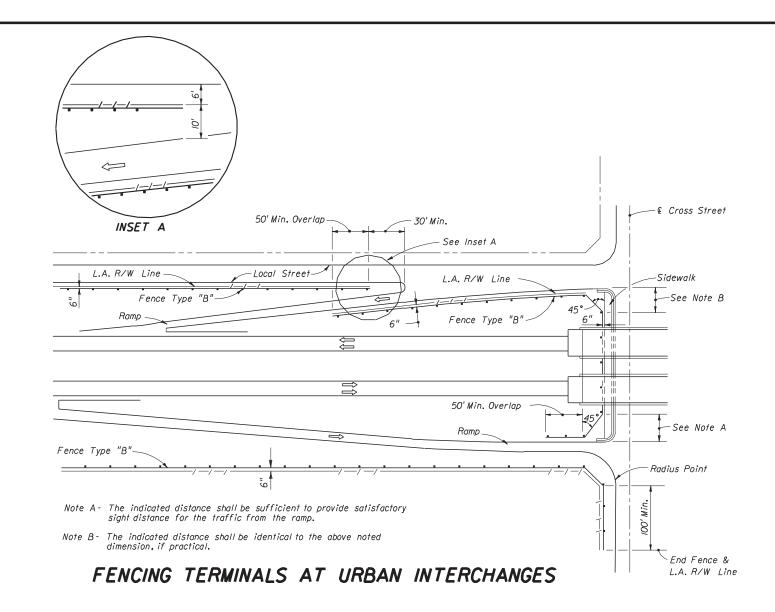


FENCING TERMINALS AT RURAL INTERCHANGES





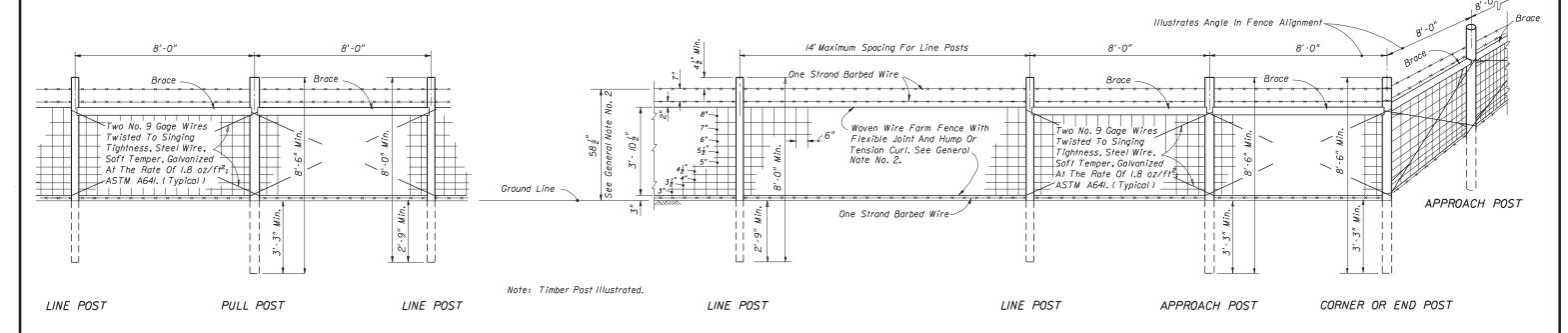
FENCING TERMINALS AT RETAINING WALLS



STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

FENCE LOCATION

	Names	Dates	Approve	d By	D-71
Designed By	HEW	02, 65		State Roadway	Design En(gi)neer
Drawn By	I!FW	02/65	Revision	Sheet No.	Index No.
Checked By	RLO	02/65	00	2 of 2	450



GENERAL NOTES

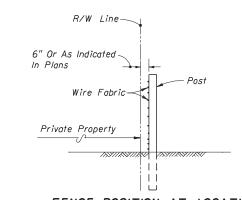
- I. This fence to be provided generally in rural areas. For supplemental information see Section 550 of the FDOT Specifications.
- 2. Fabric shall be woven wire, either galvanized steel, meeting the requirements of ASTM All6, No. 9 Farm, Design Number 1047-6-9, with Class 3 zinc coating, or aluminum coated steel, meeting the requirements of ASTM A584, No. 9 Farm, Design Number 1047-6-9, with a minimum coating weight of 0.40 oz./ft2. For additional information see payment note below.
- 3. Fence shall be installed with wire side to private property except on horizontal curves greater than 3° the fence shall be installed so as to pull against all posts.
- 4. Posts may be either timber, steel, recycled plastic or concrete. Unless a specific post material is called for in the plans, the Contractor may elect to use either a single material or a combination of timber, steel, recycled plastic or concrete materials. Line posts of one material may be used with corner, pull and end post assemblies of a different material. Line posts of only one optional material and pull post assemblies of only one optional material will be permitted between corner and end post assemblies. Within individual corner and end post assemblies only one optional material will be permitted.
- 5. Timber line posts are to be minimum 4" diameter. Timber corner, pull, approach and end posts are to be minimum 5" diameter. Timber braces are to be minimum 4" diameter.
 - (A) Staples for line posts to be $l_{\underline{d}}^{\mu}$ minimum length; for approach, corner and pull posts $l_{\underline{d}}^{\mu}$ minimum length. At approach, corner and pull posts, staple every line wire. At line posts, staple every line wire in top half and alternate line wires in bottom half. Staples shall be driven diagonally across the line wire with the points in separate grains.
 - (B) Connections between timber posts and braces to be provided by dowels as shown in fastener details.
 - (C) Wire to be wrapped and tied, as shown in the splice details, at the following locations:
 - (a) All end posts (b) Corner post, including the assemblies at vertital breaks of 15° or more.
 - (c) Pull posts where the wire is not spliced and pulled through the assembly; see General Note 16.
- 6. Steel posts and braces shall be standard steel posts, galvanized at the rate of 2 oz/ft², together with necessary
 - hardware and wire clamps and meeting the following requirements:

 (A) Line posts: 8' long; 1.33 lbs./ft.; roll formed studding; anchor plate attached (23 in²).
 - (B) Approach posts: $2\frac{1}{Z}$ " x $2\frac{1}{Z}$ " x $2\frac{1}{Z}$ " angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - Pull, end and corner posts: $2\frac{1}{2}$ " x $2\frac{1}{2}$ " x $2\frac{1}{2}$ " angles, 8' long; fabricated for attaching brace; with necessary hardware, clamps, etc.
 - (D) Braces: $2'' \times 2'' \times \frac{1}{4}''$ angles with necessary hardware and fabricated for attaching to post.
 - (E) The pull, corner, approach and end posts are to be set in concrete as per detail. (Also see Note No. 13)
- 7. Recycled plastic posts shall meet the material requirements of specification Section 972 and be one of the products included on the Qualified Products List current at the time of installation. Line posts shall have a minimum section of 4" round or 4" square. Plastic posts shall not be used as corner, pull, end or approach posts unless such use specifically detailed in the plans. Plastic posts can be set by either digging and tamped backfill or by driving into full depth preformed holes $\frac{1}{4}$ " to $\frac{1}{2}$ " smaller than cross section of post. Staples for fabric and barbed wire connection to plastic line posts shall be the same size, count and location as that for timber posts.
- The Contractor, at his option, may use any suitable precast or prestressed concrete posts; however, approval by the Engineer, of posts not shown on this index, will be required prior to construction of the fence. Precast posts shall be Class I concrete. Prestressed posts shall be Class III concrete. Lengths of concrete post to be as indicated for
- 9. Aluminum post, braces and accessory framing hardware shall not be used unless the plans specifically detail their application or the Engineer specifically approves their incorporation in fence construction or repair. Aluminum framed gates are permitted as described in General Note 19.

- 10. The woven wire shall be attached to steel and concrete posts by a minimum of five tie wires. The single wire ties shall be applied to the top, bottom and three intermittent line wires. The ends of each tie wire shall have a minimum of two tight turns around the line wire. Tie wires shall be steel wire not less than 0.120" diameter, zinc coating Class 3, soft temper, in accordance with ASTM A641.
- II. Steel Barbed Wire can be either of the following types:
 - Type I: This type shall conform to the requirements of ASTM Al2I, with two strands of 12½ gage wire; four point barbs, wire size 14 gage, twisted around both line wires; and, Class 3 coating.

 Type II: This type same as Type I except the two strand wires are twisted in alternating directions between
 - consecutive barbs.
 - Aluminum Barbed Wire shall be fabricated of two strands of O.IIO-inch wire with 0.08-inch diameter four-point barbs spaced at approximately $5\frac{1}{2}$ ", and at a maximum spacing of 6". The wire for the strands and for the barbs shall be of ASTM B2IIM Alloy 5052-H38 or equal.
- 12. The woven wire shall be stretched only until one-half the tension curl has been pulled out of the line wires.
- 13. Posts to be set by driving or digging, If by digging, the posts shall be set at the center of the hole and the soil tamped securely on all sides.
- 14. Longer posts than those indicated above may be required by the plans or for deeper installations.
- 15. Concrete bases for angular steel posts (pull, corner, end and approach) shall be Class I as specified in Section 347 except that the requirements of 347-7 shall not apply. Materials for Class I concrete may be proportioned by volume and/or by weight.
- I6. Pull post assemblies shall be installed at approximately 330' centers except that this maximum interval may be reduced by the Engineer on curves where the radius is less than 3°.
- 17. Corner post assemblies are to be installed at all horizontal and vertical breaks in fence of 15° or more.
- 18. A maximum length of 1320' of wire may be installed as a unit. For pulls through a pull post assembly the fabric shall be spliced by crimping sleeves only. Pulls through a corner post assembly will not be permitted
- 19. Unless otherwise called for in the plans gates shall be commercially available metal swing gates assembled and installed in accordance with the manufacturer's specifications as approved by the Engineer. Chain link swing gates in accordance with Index No. 452 may be substituted for metal swing gates as approved by the Engineer. Gate size is full opening width whether single leaf or double leaves. Payment for gates shall include the gate, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, Type A, EA.
- 20. For construction and pay purposes assemblies are defined as follows: End post assemblies shall consist of: one end post, one approach post, two braces, four diagonal tension wires and all necessary fittings and hardware. Pull post assemblies shall consist of: one pull post, two braces, four diagonal tension wires and necessary fittings and hardware. Corner post assemblies shall consist of: one corner post, two approach posts, four braces, eight diagonal tension wires and all necessary fittings and hardware.
- 21. This index details fencing that is constructed with farm fabric $46\frac{1}{2}$ " (47" nominal) in height and with specific ground clearance and specific barbed wire spacings, and, is to be paid for under the contract unit price for Fencing, Type A, LF. When the plans detail other combinations of materials or variation in dimensions, the fence shall be paid for under the contract unit price for Fencing, Type A, (x'Height), LF. Fencing Type A, LF, shall be inclusive of the lengths of pull, end and corner post assemblies but exclusive of gate widths. Assemblies shall be paid for as follows:

Corner Post Assemblies, FA. Pull and End Post Assemblies, EA



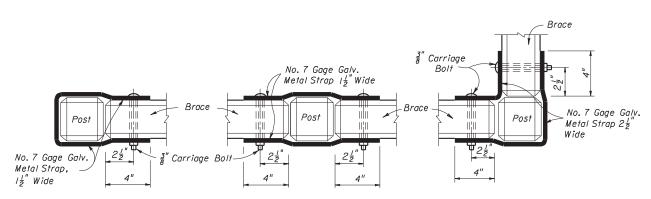
FENCE POSITION AT LOCATIONS WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

FENCE TYPE A

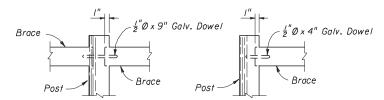
	Names	Dates	Approve	d By	♪¬ı .
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	1 of 2	451



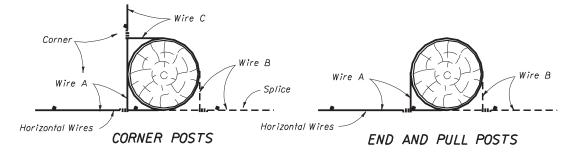
BRACE AND POST BRACE TO BRACE ON LINE BRACE TO BRACE AT CORNER

FASTENER FOR CONCRETE POST AND BRACES

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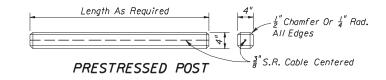


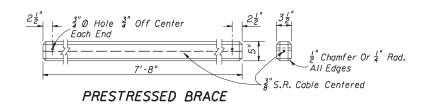
FASTENER FOR TIMBER POST AND BRACE

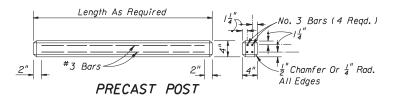


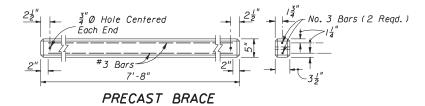
Each horizontal wire to be wrapped around corner, end and pull posts and tied to same wire. See General Notes 5 and 17. Timber post illustrated. These methods also apply to steel and concrete post illustrations.

SPLICES

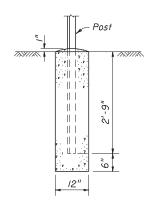








ALTERNATE CONCRETE POSTS AND BRACES



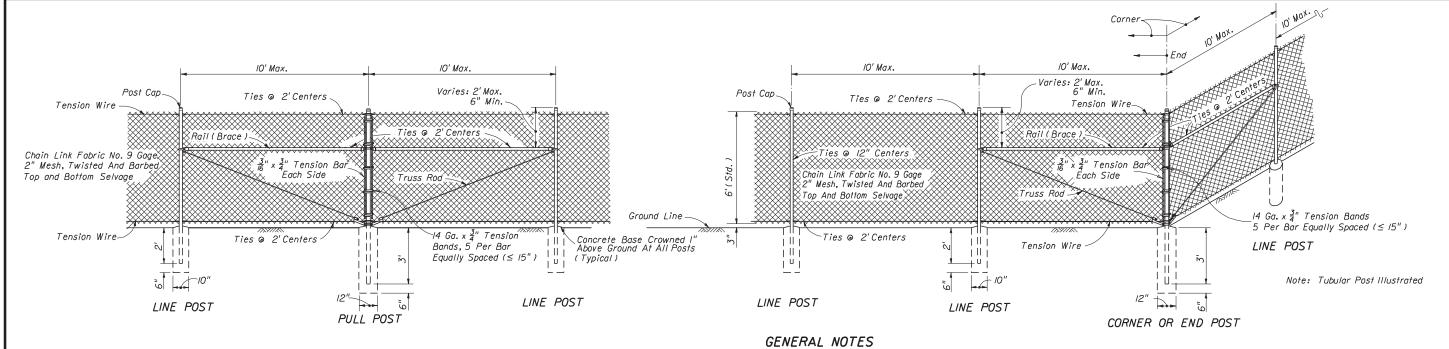
(Pull, Corner, End And Approach Posts)

CONCRETE BASE FOR ANGULAR STEEL POST



FENCE TYPE A

	Names	Dates	Approve	d By	D-71
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	2 of 2	451



- I. This fence to be used generally in urban areas.
- 2. For supplemental information refer to Sections 550 of FDOT Standard Specifications.
- 3. Chain link fabric, posts, rails, truss rods, tension wires, tie wires, stretcher bars, gates and all miscellaneous fittings and hardware shall meet the requirements of AASHTO MIBI unless otherwise specified by this index. Stipulated AASHTO and ASTM signify current reference.
- 4. Fence Component Options:
 - A. Line post options:
 - (1) Galvanized steel pipe, Schedule 40 I¹/₂" nominal dia. zinc galvanized at the rate of I.8 oz/sf: ASTM A53 Table X 2, ASTM F1083, and AASHTO MIII.
 (2) Aluminum coated steel pipe; ASTM A53, X 2 Tables Schedule 40; I¹/₂" nominal dia., I.90" O.D.;
 - coated at the rate 0.40 oz/sf: AASHTO MIII.
 - (3) Aluminum alloy pipe 2" nominal dia.: ASTM B24I or B22I, Alloy 6063, T6.
 - (4) Steel H-Beam- I_R^{7} x I_R^{5} ": Zinc Galv. I.8 oz/sf: AASHTO MIII and Detail.
 - (5) Aluminum alloy H-Beam- I_8^{7} " x I_8^{5} ": Detail.
 - (6) Steel C Ig" x Ig": Galv.: I.8 oz/sf zinc: AASHTO MIII; or, 0.9 oz/sf zinc- 5% aluminum-mischmetal: ASTM F1043 and Detail.
 - (7) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry 2" $0.0.1\frac{1}{2}$ " NPS, 1.900" dec. equiv., 0.120" min, wall thick, and min. wt. 2.28 lb/ft; with ASTM FI043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15 μ q/in² min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.
- B. Corner, end, and pull post options:
 - (I) Galvanized steel pipe, Schedule 40- 2" nominal dia. zinc galvanized at the rate of I.8 oz/sf: ASTM A53 Table X 2, ASTM F1083, and AASHTO MIII.
 - (2) Aluminum coated steel pipe; ASTM A53 steel, X 2 Tables Schedule 40; 2" nominal dia., 2.375" O.D.; coated at the rate 0.40 oz/sf: AASHTO MIII.
 - (3) Aluminum alloy pipe $2\frac{1}{2}$ " nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
 - (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry $2\frac{1}{2}$ " O.D., 2" NPS, 2.375" dec. equiv., 0.130" min. wall thick. and min. wt. 3.1/7 lb/ft; with ASTM FIO43 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15 µg/in² min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM F1043.
- C. Rail options:

- (1) Galvanized steel pipe, Schedule 40- $l_A^{"}$ nominal dia. zinc galvanized at the rate of 1.8 oz/sf: ASTM A53 Table X 2, ASTM FI083, and AASHTO MIII.
- (2) Aluminum coated steel pipe; ASTM A53 steel, X 2 Tables Schedule 40; l_4^H nominal dia., 1.660" O.D.; coated at the rate 0.40 oz/sf: AASHTO MIII.
- (3) Aluminum alloy pipe l_4^{l} " nominal dia.: ASTM B241 or B221, Alloy 6063, T6.
- (4) Resistance welded steel pipe; 50,000 psi min. yield strength ASTM A569/A569M, A653/A653M or undepleted stock of discontinued A446/A446M base materials; ASTM F669 Group IV (Alternative Design); fence industry $1_8^{\rm fi}$ (D.), $1_4^{\rm fi}$ NPS, I.660" dec. equiv., 0.III" min. wall thick. and min. wt. I.836 Ib/ft; with ASTM F1043 metric equivalent internal coating Types A, B, C or D and external coating Types A, B, or C; the chromate conversion coating of external Type B shall have a thickness of 15 μ g/in² min. and the polymer film topcoat shall have a thickness of 0.0003" min.; internal and external coatings are not restricted to the combinations of Table 2, ASTM FI043

- D. Chain link fabric options (2" mesh with twisted and barbed selvage top and bottom for all options except as described in Note No. 10):
 - (I) AASHTO MIBI Type I Zinc Coated Steel, No. 9 gage (coated wire diameter), coated at the rate of 1.8 oz/sf (MI81 Class D 2.0 oz/sf modified to 1.8 oz/sf).
 - (2) AASHTO MI8I Type II Aluminum Coated Steel, No. 9 gage (coated wire diameter), coated at the rate of 0.40 oz/sf.
 - (3) AASHTO MIBI Type IV Polyvinyl Chloride (PVC) Coated Steel, No. 9 gage (coated core wire diameter), core wire-zinc coated steel. PVC coating: MI8I Class A (either extruded or extruded and bonded) or Class B (bonded). See table right.
- E. Tension wire options:
 - (1) Steel wire No. 7 gage zinc galvanized at the rate of 1.2 oz/sf: AASHTO MI81.
 - (2) Aluminum alloy wire with a diameter of 0.1875" or larger conforming to the requirements of ASTM B2II, Allov 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
 - (3) Aluminum coated steel wire No. 7 gage coated at the rate of 0.40 oz/sf: AASHTO MI81.
- F. Tie wire and hog ring options:
 - (1) Steel wire No. 9 gage zinc galvanized at the rate of 1.2 oz/sf.
 - (2) Aluminum alloy wire with a diameter of 0.1443"or larger conforming to the requirements of ASTM B2II, Alloy 5056 Temper H38, or, Alclad Alloy 5056 Temper H192.
- Aluminum coated steel wire No. 7 gage coated at the rate of 0.40 oz/sf. 5. Unless a specific material is called for in the plans the Contractor may elect to use either a single type
- of material or a combination of material types from the component options listed above. Combinations of optional materials are restricted as follows: (a) Only one fabric optional material will be permitted between corner and/or end post assemblies. (b) Only one line post optional material will be permitted between corner and/or end post assemblies. (c) Pull post assemblies shall be optional materials identical to either the line post optional material or the corner and end post assembly optional material; but, pull post assemblies shall be the same optional material between any set of corner and/or end post assemblies.
- 6. Concrete for bases shall be either Class I concrete or 'Sackcrete' premix. Class I concrete shall be as specified in Section 347 of the Standard Specifications except the requirements contained in 347-7 shall not apply. Materials for Class I concrete may be proportioned by volume and/or by weight.
- 7. Line posts are to be set in concrete as detailed above or by the following methods:
 - (a) In accordance with special details and/or as specifically described in the contract plans and specifications.
 - (b) In accordance with ASTM F567 Subsections 4.4 through 4.7 and 4.9 and 4.10 as approved by the Engineer.
 - (c) In soils that are firm, well drained and suitable for full stable embedment any of the optional steel posts may be driven in locations approved by the Engineer. Driving will not be permitted in sandy soils. Driven posts shall be set to a minimum depth of 3' for fences up through 6' in height, and, for each I' of fence height above 6' the posts shall be set an additional 6" in depth. Posts shall be protected to prevent damage from driving. Damaged posts shall be repaired or removed and replaced as directed by the Engineer without additional cost to the Department.
 - (d) Posts mounted on concrete structure or solid rock shall be mounted in accordance with the base plate detail "Fence Mounting On Concrete Endwalls And Retaining Wall", Sheet 2; or, by embedment in accordance with ASTM F567 Subsection 4.5.

End, pull and corner post assemblies shall be set in concrete as detailed above for all soil conditions other than solid rock. Posts within assemblies that are located on concrete structures or solid rock shall be set by base plate or by embedment as prescribed under (b) above for line posts.

Line and assembly posts set in concrete bases shall be set an additional 3" in depth for each I' of fence height greater than 6'.

- 8. Pull posts shall be used at breaks in vertical grades of 15° or more, or at approximately 350' centers except that this maximum interval may be reduced by the Engineer on curves where the curve is greater than 3°.
- 9. Corner posts are to be installed at all horizontal breaks in fence at 15° or more and as required at vertical breaks over 15° as determined by the Engineer.
- 10. When fence has an installed top of fabric height less than 6', knuckled top and bottom selvages shall be used unless the plans specifically identify locations for twisted selvage fabrics.
- II. Unless sliding gates or special gates are called for in the plans, all gates shall be chain link swing gates meeting the material requirements described above as approved by the Engineer. Payment shall include the gates, single or double, all necessary hardware for installation and any additional length and/or size for posts at the opening. Gates shall be paid for under the contract unit price for Fence Gates, Type B, EA.
- 12. Line posts, tension wires, chain link fabric, tie wires, Class I concrete, and all miscellaneous fittings and hardware to be included in the cost for Fencing Type B, LF. The standard 6' high fence shall be paid for under the contract unit price for Fencing Type B, LF. Fence having other height, line components and/or barbed wire attachments shall be paid for under the contract unit price for Fencing Type B (__), LF.

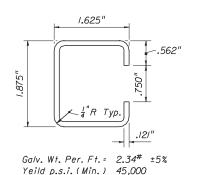
Corner post assemblies shall consist of one corner post, two braces, two truss rods, and all necessary fittings and hardware as detailed above and shall be paid for under the contract unit price for Corner Post Assembly (Type B Fence), EA.

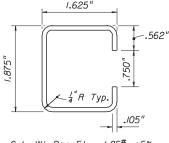
End post assemblies shall consist of one end post, one brace, one truss rod and all necessary fittings and hardware as detailed above and shall be paid for under the contract unit price for End Post Assembly (Type B Fence), EA.

TYPE IV VINYL COATED FABRIC										
Specification Section 966 And AASHTO MIBI Table 4 Redefined As Follows										
Specified Diameter Minimum Weight PVC Thickness Range										
Of Me	tallic C ore Wir	oated		Of Zinc Coating M/81 Class A (Extruded Or Extruded And Bonded Coating)				ass B Coating)		
in.	mm	gage	oz/sf	g/m²	in.	mm	in.	mm		
0.148	3.77	9	0.30	92	0.015 to 0.025	0.38 to 0.64	0.006 to 0.010	0.15 to 0.25		

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION **FENCE** TYPE B

	Names	Dates	Approve	d By	A-71
esigned By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	1 of 2	452



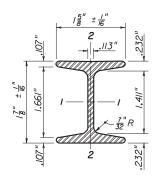


Galv. Wt. Per. Ft. = 1.85# ±5% Yeild p.s.i. (Min.) 45,000

STANDARD WALL

THINWALL

OPTIONAL "C" LINE POST

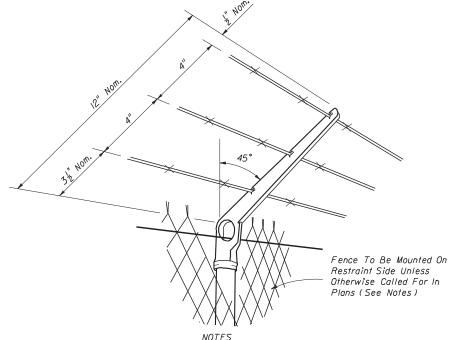


	ST	EEL	ALUM	INUM
Area (Sq. In.)	72	24	7.	24
Weight (Lb./Ft.)	2.72 ±5	% (Galv.)	0.9	1 ±5%
Surface Area (SF/Ft.)	.77	76	.77	76
Tensile Strength (psi Min.)	80,0	000	30,	000
Yielding Point (psi Min.)	48,0	000	25,	000
	Ax	res	Ax	es
	1-1	2-2	1-1	2-2
Moment Of Inertia	.428	.101	.428	.101
Section Modulus	.456	.124	.456	.124
Rad. Of Gyration	.779	.373	.779	.37

R/W Line 6" Or As Indicated In Plans Wire Fabric Private Property

FENCE POSITION AT LOCATIONS
WITHOUT FRONTAGE ROADS

(REFER TO DETAIL PLANS FOR FENCE POSITION AT LOCATIONS WITH FRONTAGE ROADS)



Attachments to be used only when called for in the plans.

Attachments to extend in direction of restraint. Unless otherwise called for in plans, direction of restraint will be as follows:

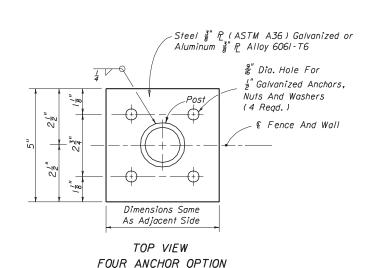
- (a.) Outward on limited access right of way line.
- (b.) Outward on controlled access right of way line.
- (c.) Outward from utilities and hazardous facilities located within highway right of way.
- (d.) Outward from lateral ditches, outfalls, retention basins, canals, borrow areas and similar support facilities.
- (e.) Inward on pedestrian ways.

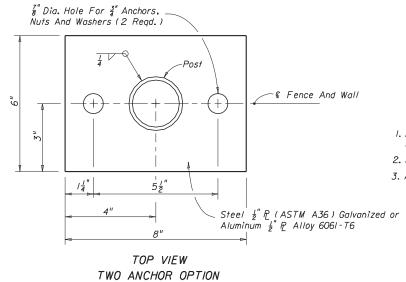
The cap-arm shall be designed to provide a drive fit over the top of posts and to exclude moisture in posts with tubular sections.

Attachments to be paid for under the contract unit price for Fencing, Type B (With Barb Wire Attachment) LF.

BARB WIRE ATTACHMENT

OPTIONAL IT X IT H-BEAM LINE POST





BASE PLATE AND ANCHOR NOTES:

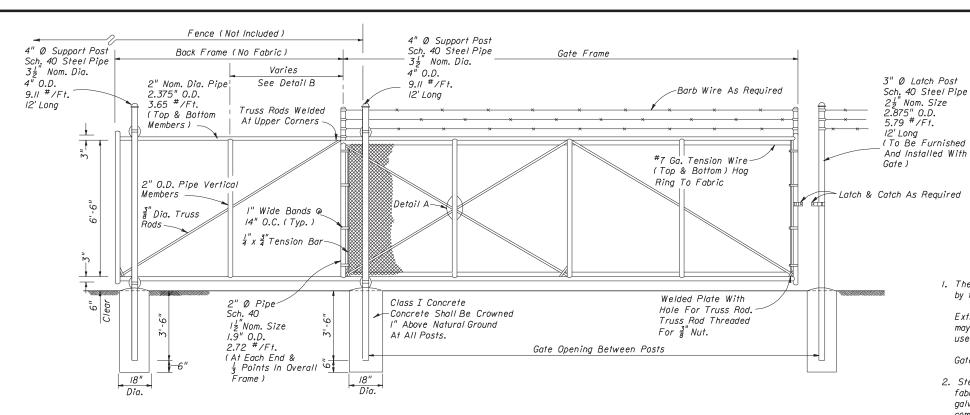
- I. Base plate identical for line, pull, end and corner posts and shall be considered an integral part of the respective posts for basis of payment.
- 2. Post to be plumbed by grout shim under base plate.
- 3. Anchors (Galvanized Steel):
 - I2" Cast In Place, $IO_2^{L''}$ Embedment: Headed Bolts, U-Bolts or Cluster Plates.
 - 8" Adhesive Anchors, 6" Min. Embedment, *
 - *Adhesive anchors shall be headless anchor bolts set in drilled holes with an Adhesive Material System in accordance with Specification Sesctions 416 and 937; drilled holes shall be $\frac{t}{8}$ " larger in diameter than the anchor bolt.
 - Expansion Bolts Not Permitted.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

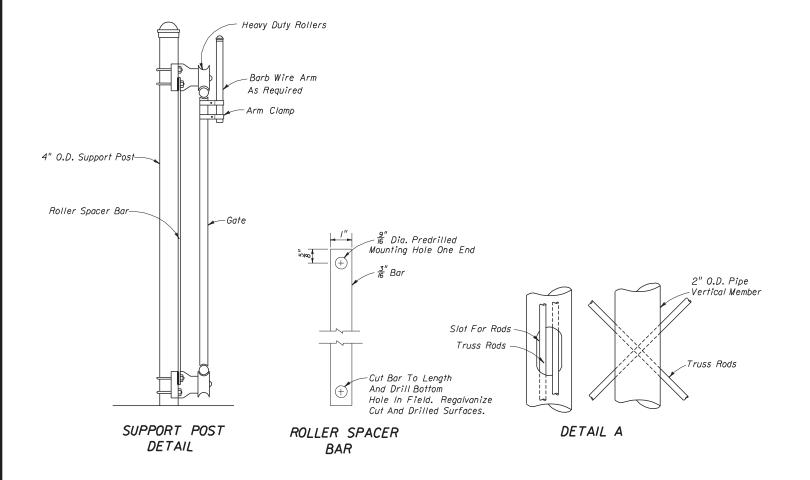
FENCE TYPE B

	Names	Dates	Approve	d By	A-71
Designed By				State Roadway	Design Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	2 of 2	452

FENCE MOUNTING ON CONCRETE ENDWALL AND RETAINING WALLS



FRONT ELEVATION

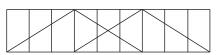


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GATE OPENING	GATE FRAME	BACK FRAME
12'	12'-3"	6'
16'	16'-3"	8'
20'	20' - 3"	10'
24'	24'-3"	12'



TYPICAL FRAME - 12', 16' & 20' Opening



DETAIL B

TYPICAL FRAME - 24' Opening

GENERAL NOTES

 The Contractor may substitute any equivalent cantilever slide gate approved by the Engineer.

Extruded, rolled or formed components that provide equal strength and stability may be used in lieu of the pipe components shown; and, internal rollers may be used in lieu of the external roller units shown.

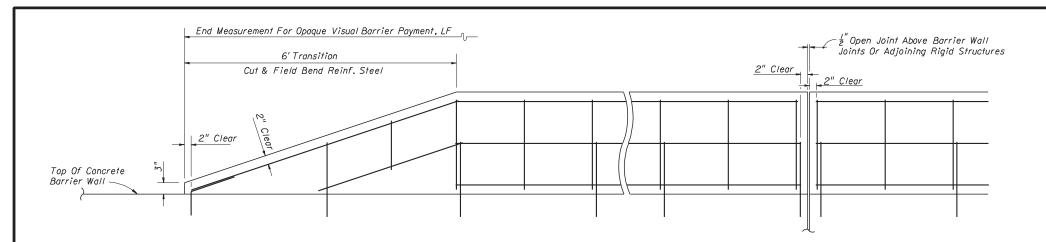
Gate components shall meet or exceed the protective coatings specified on Index No. 452.

- 2. Steel gate frame shall be fabricated prior to galvanizing, except that truss rods may be fabricated following frame galvanizing provided surfaces damaged during welding are galvanized in accordance with Section 24 of AASHTO M36; or, fabricated from pipe components with protective coating meeting the requirements of Index No. 452 that are tolerant of welding (low burn back), and a protective coating applied to the weld and damaged pipe surfaces that is equivalent to the protective coating of the fabricated pipe stock.
- 3. All fabric shall be knuckled top and bottom selvages.
- 4. Cost of all gate components shall be included in the contract unit price for Sliding Fence Gate (Cantilever), EA.

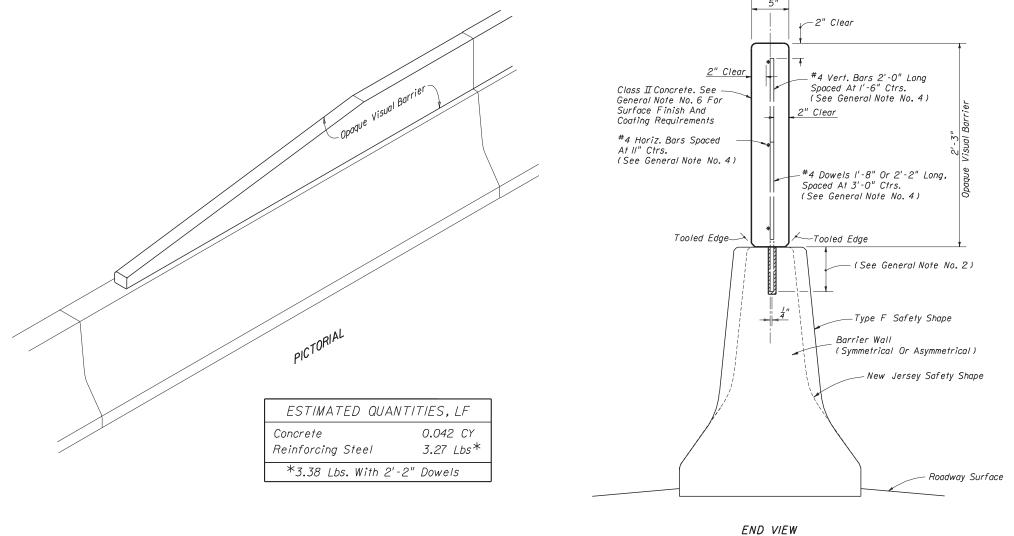
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

CANTILEVER SLIDE GATE TYPE B FENCE

	Names	Dates	Approve	d By	₽¬/
Designed By				State Roadway	Design Engineer
Drawn By	HDD	9/78	Revision	Sheet No.	Index No.
Checked By	LMF	9/78	00	I of I	453



ELEVATION OF REINFORCEMENT AND DOWELING



GENERAL NOTES

- I. The opaque visual barrier is intended to function as a visual screen, and is not intended to resist vehicle impact loads nor to restrain, contain or restrict vehicles or cargo. The barrier is designed to withstand zone wind loading and strikes by light debris; and, designed to yield to exceptional strikes by vehicles or cargo, and to contain ruptured segments of the screen when yielding to such strikes.
- 2. When the opaque visual barrier is constructed on an existing barrier wall, dowels shall be l'-8" in length, embedded 6" into the barrier wall and set with an approved chemical grout. Embedment holes shall be $\frac{5}{8}$ " diameter, drilled to a depth $\frac{1}{4}$ " below the tip of the dowel unless greater depth is required to accept manufactured grout capsules.

When the opaque visual barrier is constructed in conjunction with project concrete barrier walls, dowels may be set as described above, in either the drilled or preformed the drilled or preformed holes; or, placed when the barrier wall is cast. For dowels that are placed when the wall is cast, the dowel shall be 2'-2" in length and embedded to a depth of 12".

3. For both double and single faced concrete barrier walls the opaque visual barrier is to be located in the center of the top of the wall.

For single faced barrier walls that are constructed around other vertical structure, the opaque visual barrier shall follow the alignments of only one of the walls and be centered atop that wall.

For dual median barrier walls that follow differential profiles, the opaque visual barrier shall be constructed atop the wall with the higher elevation, unless conditions dictate otherwise. Lateral transitions or end overlaps for opaque visual barriers that alternate between dual walls shall be detailed in the plans.

For median barrier walls that are divided when connecting to separated bridges, the opaque visual barrier shall be constructed atop the approach side barrier wall, unless differential profiles dictate locating the opaque visual barrier on the departure side barrier wall.

Opaque visual barriers to be located on capped fills between dual barrier walls shall be detailed in the plans.

- 4. In lieu of the reinforcement shown the Contractor may substitute welded wire fabric equal to or better than that shown, when approved by the Engineer. Details shall be submitted with requests for substitution.
- 5. The Contractor may construct contiguous precast concrete panels in lieu of the cast-in-place opaque screen when approved by the Engineer. Panel design and method for anchorage to the barrier wall shall be detailed by shop drawings when requesting the Engineers approval.

The Contractor may construct the opaque screen monolithically with the barrier wall, however, the screen design shall not be modified so as to cause the wall to be dynamically active from strikes on the screen; see design considerations in Note No. I above.

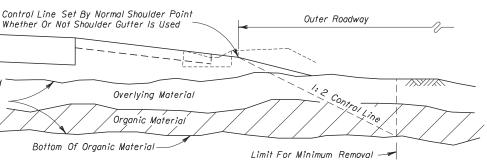
- 6. Exposed concrete surfaces shall have a Class 3 surface finish in accordance with Section 52l of the Standard Specification, unless other finish called for in the plans. The surfaces shall have a Class 5 Applied Finish Coating in accordance with Section 400 only when called for in the plans.
- 7. Payment for opaque visual barrier shall be full compensation for concrete, reinforcement, dowels, casting, placement, drilling, grouting, tooling, finishing and work incidental thereto, and shall be paid for under the contract unit price for Opaque Visual Barrier (Concrete) (2'-3" Height). LF.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

OPAQUE VISUAL BARRIER

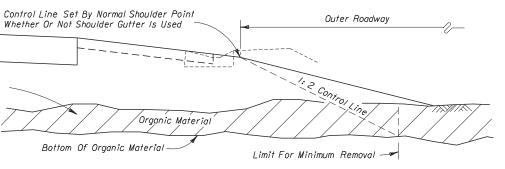
	Names	Dates	Approve	d By	1-7
Designed By	DCB/JVG	9/87		State Roadway	Design Engineer
Drawn By	JBW	9/87	Revision	Sheet No.	Index No.
Checked By	DCB, JVG	9/87	00	1 of 1	461

Remove Overlying Material And Organic Material Within The Limits Shown And Backfill In Accordance With Index No. 505, Unless Otherwise Called For In The Plans Or Directed Otherwise By The District Geotechnical Engineer; The Limits Include Full Median Width When Applied To Divided Facilities With Median Widths Up To 64': When Median Width Is Greater Than 64' And For Bifurcated Roadways The Organic Material Removal Limits Will Be Set By A 1:2 Control Line Complimentary To The Outer Roadway That Will Accomodate One Future Median Lane On Each Roadway Unless Specified Otherwise By The Plans.



WITH OVERBURDEN - HALF SECTION

Remove Overlying Material And Organic Material Within The Limits Shown And Backfill In Accordance With Index No. 505, Unless Otherwise Called For In The Plans Or Directed Otherwise By The District Geotechnical Engineer; The Limits Include Full Median Width When Applied To Divided Facilities With Median Widths Up To 64': When Median Width Is Greater Than 64' And For Bifurcated Roadways The Organic Material Removal Limits Will Be Set By A 1:2 Control Line Complimentary To The Outer Roadway That Will Accomodate One Future Median Lane On Each Roadway Unless Specified Otherwise By The Plans.

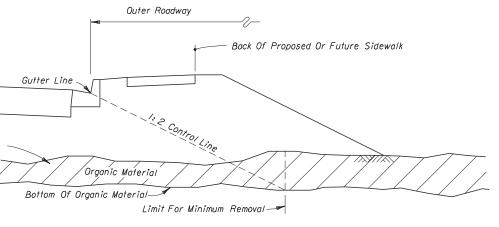


WITHOUT OVERBURDEN - HALF SECTION

IN RURAL CONSTRUCTION

Outer Roadway Back Of Proposed Or Future Sidewalk Remove Overlying Material And Organic Material Within The Limits Shown And Gutter Line Backfill In Accordance With Index No. 505, Unless Otherwise Called For In The Plans Or Directed Otherwise By The District Geotechnical Engineer; The Limits Include Full Median Width When Applied To Divided Overlying Material Facilities With Median Widths Up To 64'. When Median Width Is Greater Than 64' Organic Material And For Bifurcated Roadways The Organic Material Removal Limits Will Be Set By A 1:2 Control Line Complimentary To The Bottom Of Organic Material Outer Roadway That Will Accomodate One Limit For Minimum Removal -Future Median Lane On Each Roadway Unless Specified Otherwise By The Plans.

Remove Overlying Material And Organic Material Within The Limits Shown And Backfill In Accordance With Index No. 505, Unless Otherwise Called For In The Plans Or Directed Otherwise By The District Geotechnical Engineer; The Limits Include Full Median Width When Applied To Divided Facilities With Median Widths Up To 64'; When Median Width Is Greater Than 64' And For Bifurcated Roadways The Organic Material Removal Limits Will Be Set By A 1:2 Control Line Complimentary To The Outer Roadway That Will Accomodate One Future Median Lane On Each Roadway Unless Specified Otherwise By The Plans.



WITHOUT OVERBURDEN - HALF SECTION

WITH OVERBURDEN - HALF SECTION

IN URBAN CONSTRUCTION

REMOVAL OF ORGANIC MATERIAL

GENERAL NOTES

- unless otherwise shown on the plans.
- 2. Utilization of excavated materials shall be in accordance with Index No. 505.
- 3. Where organic or plastic material is undercut, backfill shall be made of suitable material in accordance with Index No. 505, unless otherwise shown on the plans.
- 4. The term "Plastic Material" used in this index in conjunction with removal of plastic soil is as defined under soil classifications for Plastic (P) and High Plastic (H) on Index No. 505.
- 5. The term "Organic Material" as used on this index is defined as any soil which has an average organic content greater than five (5.0) percent, or an individual organic content test result which exceeds seven (7.0) percent. Organic material shall be removed as shown on this index and the plans unless directed otherwise by the District Geotechnical Engineer.

Average organic content shall be determined from the test results from a minimum of three randomly selected samples from each stratum. Tests shall be performed in accordance with FM I-T267 on the portion of a sample passing the No. 4 sieve.

- I. All details shown on this index for removal of organic and plastic materials apply 6. The normal depth of side ditches shall be 3.5' below the shoulder point except in special cases.
 - 7. In municipal areas, where underdrain is to be constructed beneath the proposed pavement, the grade of the underdrain filter material will not extend above the bottom of the stabilized section of the subgrade. Gradation of the filter material shall conform to FDOT specifications. Minimum grade on underdrain pipe shall be 0.2%.
 - 8. See Index No. 506 for miscellaneous earthwork details.

DESIGN NOTES

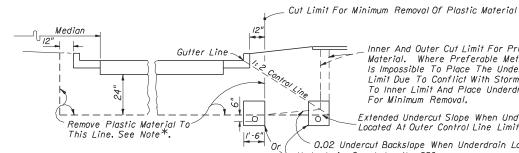
- I. At locations where organic material or other soft soil deposits persists to such depth that removal is impractical, the construction of a geosynthetic foundation over those soils should be considered. The Engineer of Record should request guidance from the District Geotechnical Engineer and make a geosynthetic foundatuion design in accordance with Index No. 501 when pursuing geosynthetic alternates.
- 2. The designer shall take into consideration the expectancy of roadway widening to the outside, and where widening is anticipated specify in the plans the limits of removal of organic and plastic materials necessary to accommodate anticipated widening.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

REMOVAL OF ORGANIC AND PLASTIC MATERIAL

	Names	Dates	Approved By								
Designed By	GEOTECH	9/93	State Geotechnical Engineer								
Drawn By	нкн	9/93	Revision Sheat No. Index No.								
Checked By	BTD/FLS	9/93	00 10f 2 500								

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Inner And Outer Cut Limit For Preferable Removal Of Plastic Material. Where Preferable Method Of Removal Governs And It Is Impossible To Place The Underdrain At The Outer Cut Limit Due To Conflict With Storm Sewer Mains, Remove To Inner Limit And Place Underdrain At Location Shown For Minimum Removal.

Extended Undercut Slope When Underdrain Located At Outer Control Line Limit

0.02 Undercut Backslope When Underdrain Located At Back Of Curb Underdrain, See Index No. 286 Minimum Grade On Underdrain Pipe Shall Be 0.2%.

HALF SECTION

NOTES: Refer to roadway cross sections to determine whether minimum or preferable removal is used.

*Where frequency of median breaks indicates that it is impractical to leave plastic material in the median, the designer may elect to indicate total removal of this material. If during construction it becomes apparent, due to normal required construction procedures, that it is impractical to leave the plastic material in the median, total removal of this material shall be approved by the Engineer.

REMOVAL OF PLASTIC MATERIAL AND LOCATION OF UNDERDRAIN IN URBAN CONSTRUCTION

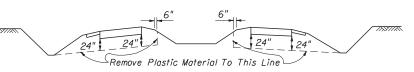


At locations where plastic material is being removed, the side ditches must be at least as deep as the undercut plane.

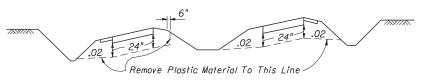
Undercut Line

Where paved side ditches are used in areas of removal of plastic material. the top of the ditch pavement must be no higher than the undercut plane.

MISCELLANEOUS DETAILS

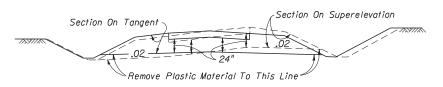


TYPICAL CUT SECTION ON TANGENT



TYPICAL CUT SECTION ON SUPERELEVATION

REMOVAL OF PLASTIC MATERIAL ON INTERSTATE FACILITIES, FREEWAYS, DIVIDED ARTERIALS AND MAJOR COLLECTORS HAVING DEPRESSED MEDIANS



TYPICAL CUT SECTION

Note: When this detail is applied to minor collectors and local facilities, the undercut may be reduced to 18".

REMOVAL OF PLASTIC MATERIAL ON DIVIDED FREEWAYS. ARTERIALS AND MAJOR COLLECTORS HAVING FLUSH MEDIANS, AND, ON UNDIVIDED ARTERIALS AND MAJOR COLLECTORS

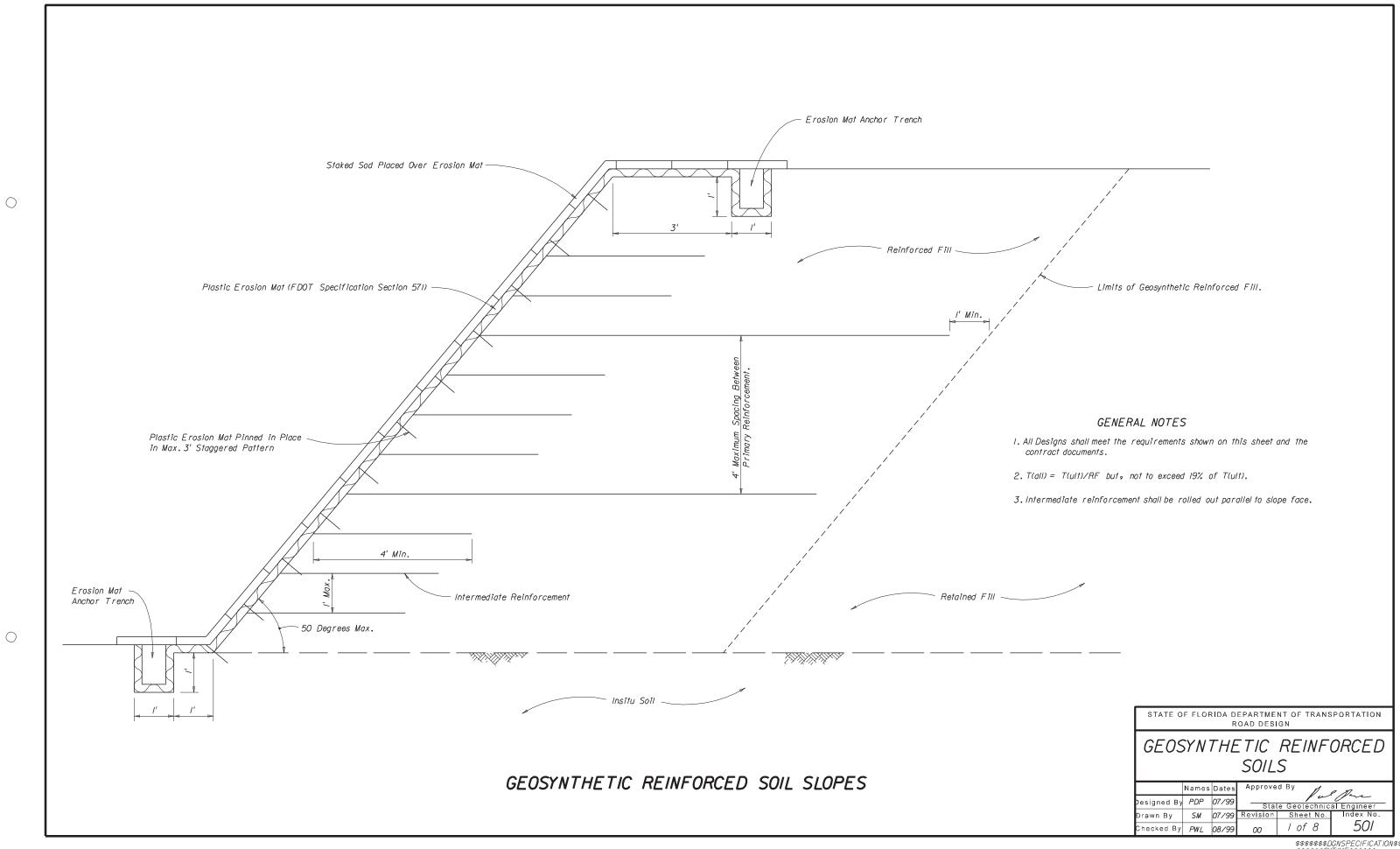
REMOVAL OF PLASTIC MATERIAL

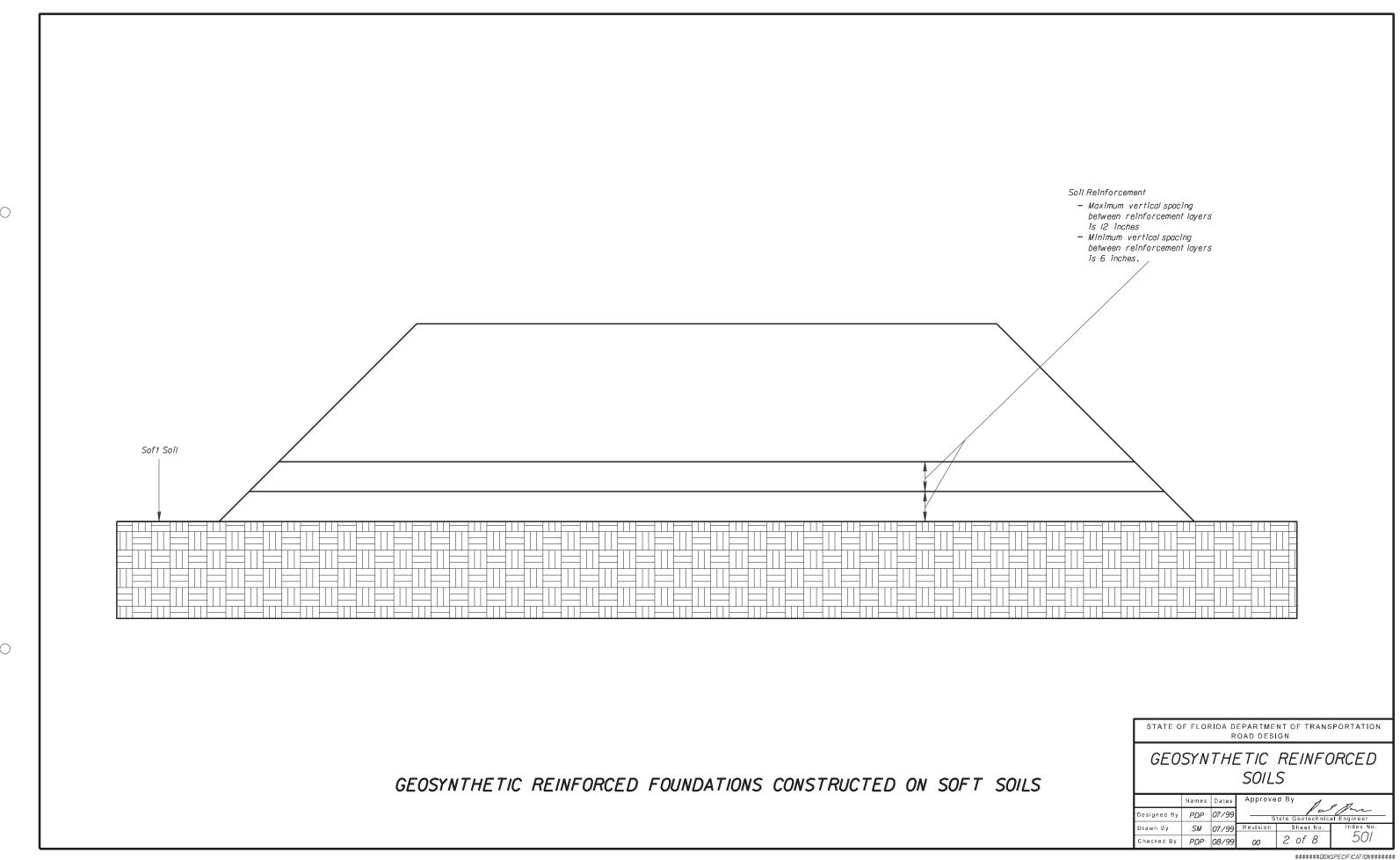
Note: For GENERAL NOTES see Sheet I.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

REMOVAL OF ORGANIC AND PLASTIC MATERIAL

	Names	Dates	Approve	. //	e 21
Designed By	KHH/WNJ.	05/91		State Geotechr	<i>U</i>
Drawn By	HKH	05/91	Revision	Sheet No.	Index No.
Checked By	JVG/WNL	05/91	00	2 of 2	500





\$\$\$\$\$\$\$DGNSPECIFICATION\$

TABLE OF WOVEN GEOTEXTILE VALUES												
P	PROPERTY	REQUIRED TEST METHOD	MIRAFI HP 370	MIRAFI HP 470	MIRAFI HP 570	MIRAFI HP 670	MIRAFI HP 770	MIRAFI HS 400	MIRAFI HS 600	MIRAFI HS 800	MIRAFI HS 1150	
Permittivit	ty (0.05 sec ^{-/} Min.)	ASTM D 4491	0.52	0.20	0.40	0.50	0.23	0.026	0.32	0.20	0.32	
1	lity (Retained 50% n Min.@ 500 hr.)	ASTM D 4355	70%	70%	70%	70%	70%	70%	70%	70%	70%	
Burst St	trength (psi)	GRI : GSI	800	1,200	1,200	1,200	1,200					
Grab St	trength (lb)	ASTM D 4632	400 x 250	380 x 350	475 x 440	650 x 450	600 x 550				· <u> </u>	
A.0).S.(in)	ASTM D 4751	0.0236	0.0335	0.0236	0.0335	0.0236	0.0118	0.0335	0.0335	0.0236	
Tensile Str	rength (lb/ft)											
on on	Ultimate		3,240	3,600	4,800	6,420	7,200	4,800	7,200	9,600	13,800	
Machine Direction	2% Ultimate		540	900	960	1,080	1,080					
ME	5% Ultimate	ASTM D 4595	1,356	1,800	2,400	2,700	3,000	1,080	2,040	3,600	4,800	
ion	Ultimate		2,700	3,600	4,800	4,800	4,800	4,800	3,600	3,600	3,600	
Cross Direction	2% Ultimate		540	1,200	1,320	1,200	1,320					
0 /0	5% Ultîmate		/ , 356	1,800	2,400	2,700	2,400	2,400				
1	e Ultimate trength (lb/ft)		14%	10%	10%	14%	12%	15%	15%	10%	12%	
6	2% strain	ASTM D 4595	27,000	45,000	48,000	54,000	54,000					
Secant Modulus @	5% strain		27,120	36,000	48,000	54,000	60,000	21,600	40,800	72,000	96,000	
Nod	IO% strain		24,000	36,000	48,000	54,000	66,000	33,600	57,600	96,000	120,000	
Seam Brea	king Strength (lb/ft) ASTM D 4884	1,440	1,800	3,000	3,600	1,200	2,400	2,400	2,400	2,400	
Punci	ture Resistance (Ib)	ASTM D 4833	180	170	190	200	220					
Tear Strength (1b)	Machine Direction	ASTM D 4833	180	130	180	250	250					
Stri	Cross Direction	ASTM D 4833	110	200	180	200	400					
	synthetic Friction	GRI: GG5, GT7	0.8	0.8	0.8	0.8	0.8	0.9	0.9	0.9	0.9	
Creep Resi	istance-T creep (Ib/ft)	ASTM D 5262						2,880	4,320	5,760	8,280	
1	eduction Factor Toreep	GRI: GG3 & GT5	5.0	5.0	5.0	5.0	5.0	1.67	1.67	1.67	1.67	
Installation Damage ($RF_{\cal C}$)	Sand	GRI: GG4 & GT7	1.25	1.25	1,15	1,15	1.15	1.3	1,25	1.2	1.15	
	Limestone		1,5	1.5	1.35	1.35	1.35	5	3.5	1.85	1.7	
Durability (RF _d)	Chemical	ASTM D 5322	1,1	IJ	I.J	1.1	1,1	1.J	1,1	1.1	1,1	
Durc (A	Biological	ASTM D1987, D3083, G21& G22	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Joint Strength (RF _j)	Mechanical	ASTM D 4595, GRI: GG4 & GT7										
Stre	Overlap	GRI: GG5 & GT6	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Approved A	Application Usage		3	3	3	3	3	3	3	3	3	

2 = Reinforcement of Foundations over Soft Soils

3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

APPROVED GEOSYNTHETIC PRODUCTS
(WOVEN GEOTEXTILES)
APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve		& Dan
Designed By	PDP	07/99	Sta	te Geotechnic	
Drawn By	PDP	08/99	Revision	Sheet No.	Index No.
Checked By	PWL	08/99	00	3 of 8	501

TABLE OF WOVEN GEOTEXTILE VALUES												
P	PROPERTY	REQUIRED TEST METHOD	MIRIFI HS 1400	MIRIFI HS 1715	MIRIFI HS 2400	MIRIFI HS 3000	MIRIFI HS 3600	AMOCO 2006	AMOCO 2016	AMOCO 2044	COMTRAC 70/70	
Permittivit	ty (0.05 sec ^{-/} Min.)	ASTM D 4491	0.20	0.32	0.02	0.02	0.02	0.05	0.70	0.15	0.20	
	ty (Retained 50% Iin. @ 500 hr.)	ASTM D 4355	70%	70%	70%	70%	70%	70%	70%	70%	70%	
Burst Stre	ength (psi)	GRI: GSI						1,000	1,100	1,500		
Grab Stren	ngth (Ib)	ASTM D 4632						3/5	3/5	600/500		
A.O.S. (in)	l	ASTM D 4751	0.0335	0.0335	0.0118	0.0118	0.0118	0.0167	0.0167	0.0236	0.0335	
Tensile Str	rength (lb/ft)											
on le	Ultimate		16,800	20,580	28,800	36,000	43,200	2,100	2,400	4,800	16,800	
Machine Direction	2% Ultimate							156	276	456		
-	5% Ultimate	ASTM D 4595	6,000	8,400	14,400	18,000	21,600	564	744	1,452	6,000	
Cross Direction	Ultimate		3,600	3,600	3,600	3,600	3,600	2,100	2,400	4,800	3,600	
Cros rect	2% Ultimate							576	660	1,380		
ia	5% Ultimate							1,104	1,404	2,604	<u> </u>	
	e Strength		14%	14%	10%	10%	10%	8%	8%	8%	14%	
6	2% strain	ASTM D 4595						7,800	/3,800	22,800		
MSBBB\$t,	5% strain		120,000	168,000	288,000	360,000	432,000	11,280	14,880	29,040	120,000	
MoSu 111	10% strain		120,000	162,000	288,000	360,000	432,000	10,440	12,480	31,200	120,000	
Seam Brea		ASTM D 4884	2,400	2,400	3,600	3,600	3,600				2,400	
Puncture F	Resistance (Ib)	ASTM D 4833						120	120	170		
StF@A§th (1b)	Machine Direction	ASTM D 4833						120	120	250		
StF	Cross Direction	ASTM D 4833			<u> </u>			120	120	250	<u> </u>	
Soil- Geos	synthetic Friction	GRI: GG5, GT7	0.9	0.9	0.9	0.9	0.9	0.65	0.65	0.65	0.9	
Creep Resi	stance-T _{cree} (lb/ft)	ASTM D 5262	10,080	12,348	17,280	21,600	21,600	600	685	1,371		
	eduction Factor Tcreep)	GRI: GG3 & GT5	1.67	1.67	1.67	1.67	1.67	3.5	3.5	3.5	1.67	
Installation Damage ($RF_{\mathcal{C}}$)	Sand	GRI: GG4 & GT7	1,15	1.15	I,I	1.1	1,1	1.10	1.05	1.05	1,15	
Installat Damagi (RF _C	Limestone		1.5	1.35	1.25	1.25	1.25	1.20	1.20	1.10	1.5	
Durability (RF _d)	Chemical	ASTM D 5322	1,1	1,1	I.J	1,1	1,1	1,1	1.1	1.1	1,1	
Durc (A	Biological	ASTM D1987, D3083, G21& G22	1.0	1.0	1.0	1.0	1.0	-			1.0	
Joint Strength (RF _j)	Mechanical	ASTM D 4595, GRI: GG4 & GT7										
Stre	Overlap	GRI: GG5 & GT6	1.0	1.0	1.0	1.0	1.0	1.2	1.2	1.2	1.0	
Approved i	Application Usage		3	3	3	3	3	3	3	3	3	

2 = Reinforcement of Foundations over Soft Soils

3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

APPROVED GEOSYNTHETIC PRODUCTS
(WOVEN GEOTEXTILES)

APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve	d By	& Jan
Designed By	PDP	07/99	Sta	te Geotechnic	0
Drawn By	PDP	08/99	Revision	Sheet No.	Index No.
Checked By	PWL	08/99	00	4 of 8	501

					TABLE	OF WOVEN GEOG	RID VALUES					
PI	ROPERTY	REQUIRED TEST METHOD	MIRIFI MG 2XT	MIRIFI MG 3XT	MIRIFI MG 5XT (Matrex 30)	MIRIFI MG 7XT	MIRIFI MG 8XT	MIRIFI MG IOXT (Matrex 60)	MIRIFI MG 18XT (Matrex 90)	MIRIFI MG 20XT (Matrex 120)	MIRIFI MG 22XT (Matrex 180)	MIRIFI MG 24XT (Matrex 240)
1	ity (Retained 50% Min. @ 500 hr.)	ASTM D 4355	70%	70%	70%	70%	70%	70%	70%	70%	70%	70%
Tensile Str	ength (lb/ft)											
υ <u>ς</u>	Ultimate		2,000	2,800	3,590	4,350	6,230	8,300	9,360	12,420	17,760	25,380
chin	2% Ultimate								<u> </u>			
Machine Direction	5% Ultimate	ASTM D 4595	1,200	1,056	1,740	2,160	2,520	3,120	4,400	5,340	7,140	10,020
s	Ultimate		2,000						<u> </u>			
Cross Direction 2%	2% Ultimate								<u> </u>			
	5% Ultimate								<u> </u>			
1	@ Ultimate e Strength		10%	10%	10%	10%	10%	10%	10%	10%	10%	10%
0	2% strain	ASTM D 4595							<u> </u>			
Secant Modulus (Ib/ft)	5% strain			21,120	34,800	43,200	50,400	62,400	88,800	106,800	142,800	200,400
Se Modu	10% strain								<u> </u>	<u> </u>		<u> </u>
-	Strength (lb/ft)	GRI : GG2						<u> </u>				
Soil- Geos	ynthetic Friction	GRI: GG5, GT7	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Creep Resis	stance-T (lb/ft) creep	ASTM D 5262	1,200	1,680	2,154	2,610	3,738	4,980	5,6/6	7,221	10,326	14,756
1 '	duction Factor	GRI: GG3 & GT5	1.67	1.67	1.67	1.67	1.67	1.67	1.67	1.67	1.67	1.67
allation image RF _C)	Sand	GRI: GG4 & GT7	1.25	1.20	1./5	1,15	1,15	1,1	1,1	1,1	1.1	I,J
Installation Damage (RF_C)	Limestone	10/11: 004 & 077	Not Recommended	1.75	1.3	1.3	1.3	1.25	1.25	1.25	1.25	1.25
Durability (RF _d)	Chemical	ASTM D 5322	1,1	1,1	1,1	IJ	1,1	1,1	1,1	1,1	1.1	1,1
Dura (R.	Biological	ASTM D1987, D3083, G21 & G22	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Joint Strength (RF,)	Mechanical	ASTM D 4595, GRI: GG4 & GT7										
Stre	Overlap	GRI: GG5 & GT6	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Approved A	Application Usage		3	3	3	3	3	3	3	3	3	3

2 = Reinforcement of Foundations over Soft Soils

3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

APPROVED GEOSYNTHETIC PRODUCTS
(WOVEN GEOGRIDS)

APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By	I Jan
Designed By	PDP	07/99	Sta	te Geotechnic	0
Drawn By	PDP	08/99	Revision	Sheet No.	Index No.
Checked By	PWL	08/99	00	5 of 8	501

	TABLE OF WOVEN GEOGRID VALUES												
PR	ROPERTY	REQUIRED TEST METHOD	SYNTEEN SF 20	SYNTEEN SF 35	SYNTEEN SF 40	SYNTEEN SF 50	SYNTEEN SF 55	SYNTEEN SF 80	SYNTEEN SF 110				
	ity (Retained 50% Min.@ 500 hr.)	ASTM D 4355	70%	70%	70%	70%	70%	70%	70%				
Tensile Stre	ength (lb/ft)												
) se	Ultimate		1,809	2,627	3,05/	3,731	3,774	5 , 583	8,126				
Machine Direction	2% Ultimate		370	462	488	791	736	1,016	1,186				
Dir	5% Ultimate	ASTM D 4595	670	725	970	922	1,159	1,273	1,684				
s	Ultîmate	7.0 1 11 2 7000	1,809	2,556	3,051	3,933	2,499	2,206	2,176				
Cross Direction	2% Ultimate		370	399	488	791	604	882	1,274				
07.0	5% Ultimate		670	583	970	922	796	/ , 563	1,581				
	@ Ultimate Strength		9.4%	14.1%	9.9%	14.2%	II.5%	14.2%	18.8%				
0	2% strain	ASTM D 4595	18 , 494	23,114	24,408	39,551	36,799	50,807	59,298				
Secant Modulus (1b/ft)	5% strain		13,397	14,499	19,404	18,432	23,174	25,459	33,676				
Nod Nod	IO% strain		15 , 206	15,234	22,089	18,432	27,137	37,910	27,380				
Junction	Strength (lb/ft)	GRI: GG2											
	inthetic Friction	GRI: GG5, GT7	0.8	0.8	0.8	0.8	0.8	0.8	0.8				
Creep Resis	stance-T _{creep} (lb/ft)	ASTM D 5262	1,005	1,523	/,525	2,201	2,265	3,182	4,026				
	fuction Factor /Tcreep)	GRI: GG3 & GT5	1.80	1.72	2.00	1.70	1.67	1.75	2.02				
Installation Damage (RF_c)	Sand	GRI: GG4 & GT7	1.05	1,15	1,15	1.08	1.08	1.08	1.08				
Instalı Dam (R.	Limestone	10/W. 00 / W 0//	1.75	1.70	1.60	1.55	1.55	1.55	/.35				
Durability (RF _d)	Chemical	ASTM D 5322	1.10	1.10	1.10	1.10	1.10	1,10	1.10				
Dura (R	Bîologîcal	ASTM D1987, D3083, G21& G22	1.10	1,10	1.10	1.10	1.10	1,10	1,10				
Joint Strength (RF _f)	Mechanical	ASTM D 4595, GRI: GG4 & GT7											
Stre	Overlap	GRI: GG5 & GT6	1.10	1.10	1,10	1.10	1.10	1,10	1,10				
Approved A	pplication Usage		3	3	3	3	3	3	3				

2 = Reinforcement of Foundations over Soft Soils

3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

APPROVED GEOSYNTHETIC PRODUCTS
(WOVEN GEOGRID)

APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

, in the second	Names	Dates	Approve	d By	I Done					
esigned By	PDP	07/99	State Geotechnical Engineer							
rawn By	PDP	08/99	Revision	Sheet No.	Index No.					
hecked By	PWL	08/99	00	6 of 8	501					

					TABLE OF	EXTRUDED GEOG	GRID VALUES					
Р	ROPERTY	REQUIRED TEST METHOD	TENSAR BX 4100	TENSAR BX 4120	TENSAR BX 4200	TENSAR BX 4220	TENSAR UX 900 HS	TENSAR UX 1100 HS	TENSAR UX 1400 HS	TENSAR UX 1500 HS	TENSAR UX 1600 HS	TENSAR UX 1700 HS
	lity (Retained 50% Min.@ 500 hr.)	ASTM D 4355		90%	90%	90%	90%	90%	90%	90%	90%	90%
Tensile St	rength (lb/ft)											
ou ou	Ultimate] [860	860	1,270	1,270	3,700	3,700	4,400	6,900	9,000	10,800
Machine Direction	2% Ultimate		240	240	370	370	840	840	1,000	1,800	2,330	2,740
Dir	5% Ultimate	ASTM D 4595	480	480	705	705	1,440	1,440	2,000	3,700	4,450	5 ,400
s	Ultîmate] [875	875	1,370	1,370						
Cross D1rection	2% Ultimate		300	300	500	500						
01/0	5% Ultimate		635	<i>63</i> 5	960	960						
	@ Ultimate e Strength						10%	10%	10%	10%	10%	10%
0	2% strain	ASTM D 4595	11,995	//•995	18,506	<i>18</i> , 506	42,015	42,015	50,000	89,993	116,518	137,012
Secant Modulus (1b/ft)	5% strain	1	9,596	9,596	14,092	14,092	28,800	28,800	40,000	73,996	89,006	108,005
Se Modu (1b	10% strain	1										
	trength (Ib/ft)	GRI: GG2	90%	90%	90%	90%	90%	90%	90%	90%	90%	90%
Soil- Geos	synthetic Friction	GRI: GG5, GT7		0.90	0.95	0.95	0.462	0.462	0.462	0.462	0.462	0.462
Creep Resi	stance-T (lb/ft)	ASTM D 5262	250	250	420	420	900	/, 350	/,850	2,800	3,700	4,650
	duction Factor	GRI: GG3 & GT5	3.5	3.5	3.27	3.27	4.J2	3.65	2.381	2.46	2.43	2.33
lation lage	Sand	GRI: GG4 & GT7	1.10	1.10	1.10	1.10	0,.10	01.10	1.10	1.10	1.10	1.10
Installation Damage ($RF_{\cal C}$)	Limestone	011; 004 & 011	1.43	1.43	1.35	1 . 35	1,25	1.25	1,20	1.20	1.20	1,20
Durability (RF _d)	Chemical	ASTM D 5322	1,1	I, I	I,I	IJ	1,1	I.J	1,1	1,1	IJ	IJ
Dura (R	Biological	ASTM D1987, D3083, G21& G22	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Joint Strength (RF _f)	Mechanical	ASTM D 4595, GRI: GG4 & GT7					1.0	1.0	1.0	1.0	1.0	1.0
Stre	Overlap	GRI: GG5 & GT6	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0	1.0
Approved ,	Application Usage		3	3	3	3	3	3	3	3	3	3

2 = Reinforcement of Foundations over Soft Soils 3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

APPROVED GEOSYNTHETIC PRODUCTS (EXTRUDED GEOGRID) APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve	d By	& Jan
Designed By	PDP	07/99	Sta	te Geotechnic	
Drawn By	PDP	08/99	Revision	Sheet No.	Index No.
Checked By	PWL	08/99	00	7 of 8	50/

	TABLE OF EXTRUDED GEOGRID VALUES										
PF	ROPERTY	REQUIRED TEST METHOD	TENAX MS 220	TENAX MS 330							
	ity (Retained 50% Min.@ 500 hr.)	ASTM D 4355	85%	85%							
Tensile Str	ength (lb/ft)										
on	Ultîmate		925	1,370							
Machine Direction	2% Ultimate		300	418							
N io	5% Ultimate	ASTM D 4595	6/5	925							
ss fon	Ultimate		1,400	2,100							
Cross Direction	2% Ultimate		445	6/6							
	5% Ultimate		890	1,340							
	@ Ultimate e Strength		12%	12%							
0	2% strain	ASTM D 4595	/5,000	20,900							
Secant Modulus ((1b/ft)	5% strain		12,330	18,500							
Se Modu (1b	10% strain										
	Strength (lb/ft)	GRI: GG2	835	1,230							
Soil- Geosy	ynthetic Friction	GRI: GG5, GT7									
	stance-T creep										
Creep Red	duction Factor	GRI: GG3 & GT5	5.0	5.0							
	Sand	GRI: GG4 & GT7	3.0	3.0							
Installation Damage ($\operatorname{RF}_{\mathcal{C}}$)	Limestone	0711. 00 7 tk 077	3.0	3.0							
Durability (RF _d)	Chemical	ASTM D 5322	2.0	2.0							
Dura (A	Bîologîcal	ASTM D1987, D3083, G21& G22									
Joint Strength (RF $_{f}$)	Mechanical	ASTM D 4595, GRI: GG4 & GT7									
Stre	Overlap	GRI: GG5 & GT6									
Approved A	Application Usage		2	2							

2 = Reinforcement of Foundations over Soft Soils

3 = Both Steepened Slopes & Reinforcement of Foundations over Soft Soils

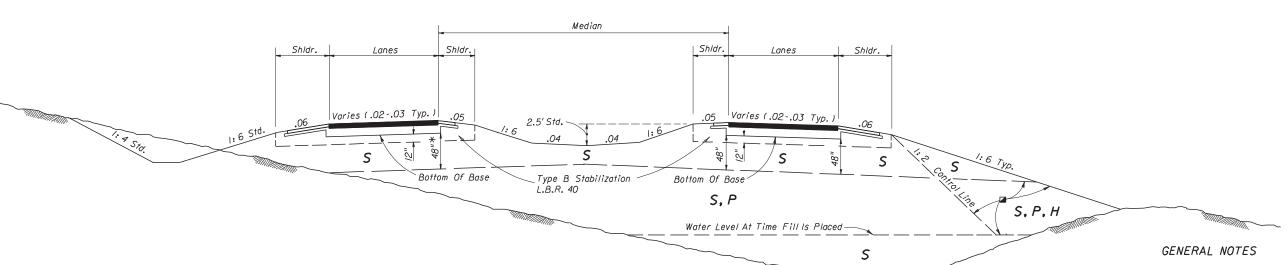
APPROVED GEOSYNTHETIC PRODUCTS

(EXTRUDED GEOGRID)

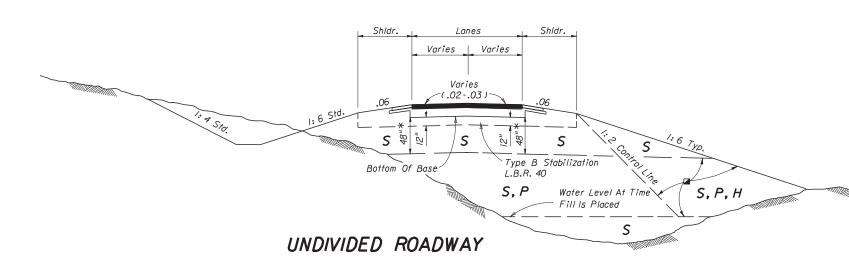
APPLICATION AND PROPERTIES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve	· //	e D.	
esigned By	PDP	07/99	State Geotechnical Engineer			
rawn By	PDP	08/99	Revision	Sheet No.	Index No.	
hecked By	PWL	08/99	00	8 of 8	501	



DIVIDED ROADWAYS



SYMBOL	<u>SOIL</u>	CLASSIFICATION (AASHTO M-145)
S	Select	A-1, A-3, A-2-4 * *
Р	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL< 50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL>50)
М	Muck	Λ - 8

Classification listed left to right in order of preference.

- See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.
- * * Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction. They may be used in the subgrade portion of the roadbed when approved by the District Geotechnical Engineer.
- * For cut sections this dimension may be reduced to 24"; see Index No. 500. For minor collectors and local facilities this dimension may be reduced to 18".

FLEXIBLE PAVEMENT

- Roadway dimensions are representative. Subgrade dimensions and control lines are standard. The
 details shown on this Index do not supersede the details shown in the plans or on Index Nos. 500
 or 506.
- Plastic (P) soils may be placed above the existing water level(at the time of construction) to within
 4 feet of the proposed base. It should be placed uniformly in the lower portion of the embankment for
 some distance along the project rather than full depth for short distances.
- 3. High Plastic (H) soils excavated within the project limits may be used in embankment construction as indicated on this index. High Plastic soils are not to be used for embankment construction when obtained from outside the project limits.
- Select (S) soils having an average organic content of more than two and one-half (2.5) percent, or having an individual test value which exceeds four (4) percent, shall not be used in the subgrade portion of the roadbed.

Select (S), Plastic (P), or High Plastic (H) soils having an average organic content of more than five (5) percent, or an organic content individual test result which exceeds seven (7) percent, shall not be used in the portion of embankment inside the control line, unless written authorization is provided by the District Geotechnical Engineer; these soils may be used for embankment construction outside the control line, unless restricted by the plans or otherwise specified in the plans, provided they can be compacted sufficiently to sustain a drivable surface for operational vehicles as approved by the Engineer.

Average organic content shall be determined from the test results from a minimum of three randomly selected samples from each stratum or stockpile of a particular material. Tests shall be performed in accordance with FM 1-T267 on the portion of a sample passing the No. 4 sieve.

5. Highly organic soils, composed primarily of partially decayed organic matter, often dark brown or black in color with an odor of decay, and sometimes fibrous, shall be designated as muck. Further, any stratum or stockpile of soil which contains pockets of highly organic material may be designated as Muck (M)

Highly organic soils shall not be used within the subgrade or embankment portion of the roadbed, with the exception of muck used as a supplement to construct a finish soil layer as described in Section 162 of the FDOT Standard Specifications.

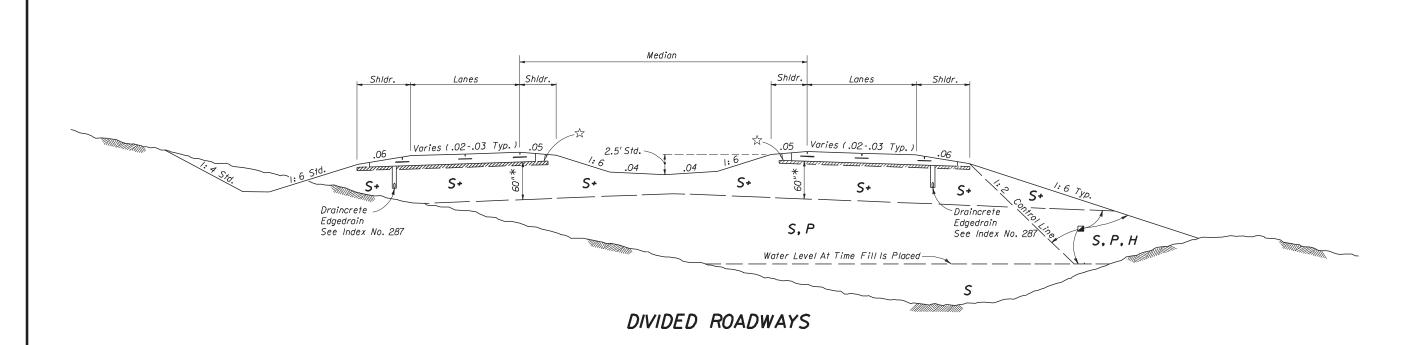
DESIGN NOTES

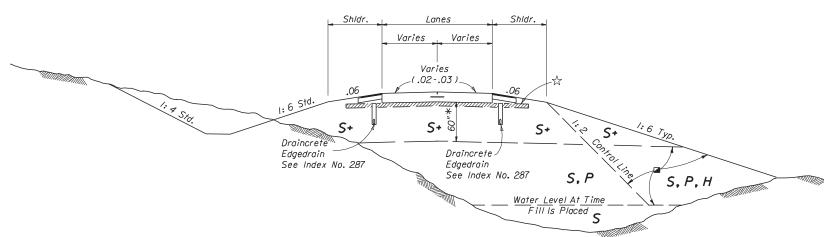
I. The designer shall take into consideration the expectancy of roadway widening to the outside, and where widening is anticipated, specify in the plans the utilization of Select(S), Plastic(P) and/or High Plastic(H) soils classified as organic material, in the embankment outside the control line.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

EMBANKMENT UTILIZATION

	Names	Dates	Approve	' //	8 12		
Designed By	GEOTECH	09/93	State Geotechnical Engineer				
Drawn By	HSD	09/93	Revision	Sheet No.	Index No.		
Checked By	BTD	09/93	00	1 of 3	505		





UNDIVIDED ROADWAY

SYMBOL	<u> 50/L</u>	CLASSIFICATION (AASHTO M-145)
S	Select	A-/, A-3, A-2-4 * *
S+	Special Select	A-3 *** With Minimum Average Lab Permeability of 5x10 ⁻⁵ cm/sec (0.14 ft./day) as per FM 1-T215
P	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL< 50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL>50)
М	Muck	A-8
Classification	on listed left to righ	t in order of preference.

- ✓ See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.
- ***When allowed by the plans, some types of A-2-4 material may be approved in writing by the District Materials Engineer.

 This material must meet the minimum lab permeability requirement, be non-plastic, and not exceed I2% passing the

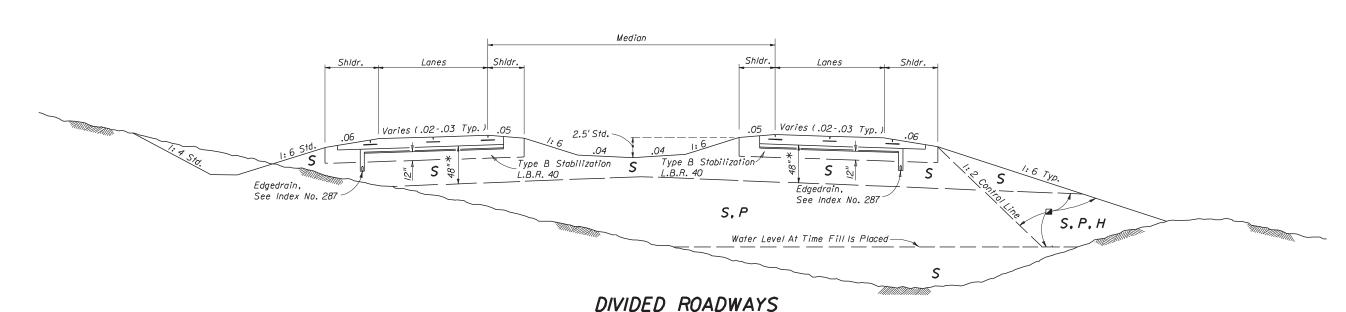
 No. 200 sieve.
- ** Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction.
- * For cut sections this dimension may be reduced to 24"; see Index No. 500. For minor collectors and local facilities this dimension may be reduced to 18".
- ☆ 3" of #57 Coarse Aggregate Mixed Into Top 6".

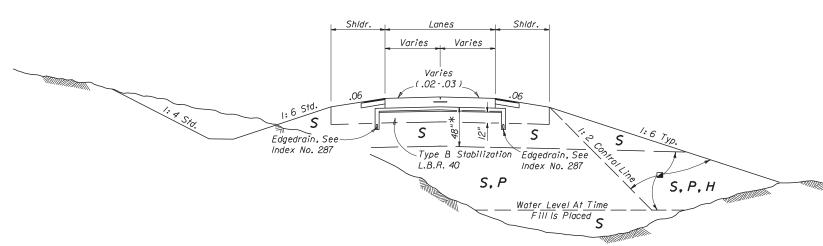
RIGID PAVEMENT - SPECIAL SELECT SOIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

EMBANKMENT UTILIZATION

	Names	Dates	Approve	. //	e D.	
Designed By	FMC	09/93	State Geotechnical Engineer			
Drawn By	HSD	09/93	Revision	Sheet No.	Index No.	
Checked By	BTD	09/93	96	2 of 3	505	





DESIGN NOTE

I. Concrete pavement is to be placed over 4" of Asphalt Treated Permeable Base (ATPB) or Cement Treated Permeable Base (CTPB) as identified in the plans. This will be placed on an aggregate separator layer using I" Type SP (Traffic C). This will be placed on a working platform using I2" of Type B Stabilization.

UNDIVIDED ROADWAY

SYMBOL	<u>SOIL</u>	CLASSIFICATION (AASHTO M-145)
S	Select	A-1, A-3, A-2-4 * *
P	Plastic	A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7 (ALL WITH LL< 50)
Н	High Plastic	A-2-5, A-2-7, A-5 Or A-7 (ALL WITH LL>50)
М	Muck	Δ-8

Classification listed left to right in order of preference.

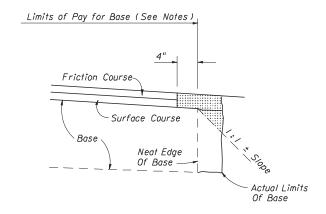
- See General Notes Nos. 4 & 5 for utilization of soils classified as organic material or muck.
- * * Certain types of A-2-4 material are likely to retain excess moisture and may be difficult to dry and compact. They should be used in the embankment above the water level existing at time of construction. They may be used in the subgrade portion of the roadbed when approved by the District Geotechnical Engineer.
- * For cut sections this dimension may be reduced to 24"; see Index No. 500. For minor collectors and local facilities this dimension may be reduced to 18".

RIGID PAVEMENT - TREATED PERMEABLE BASE

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

EMBANKMENT UTILIZATION

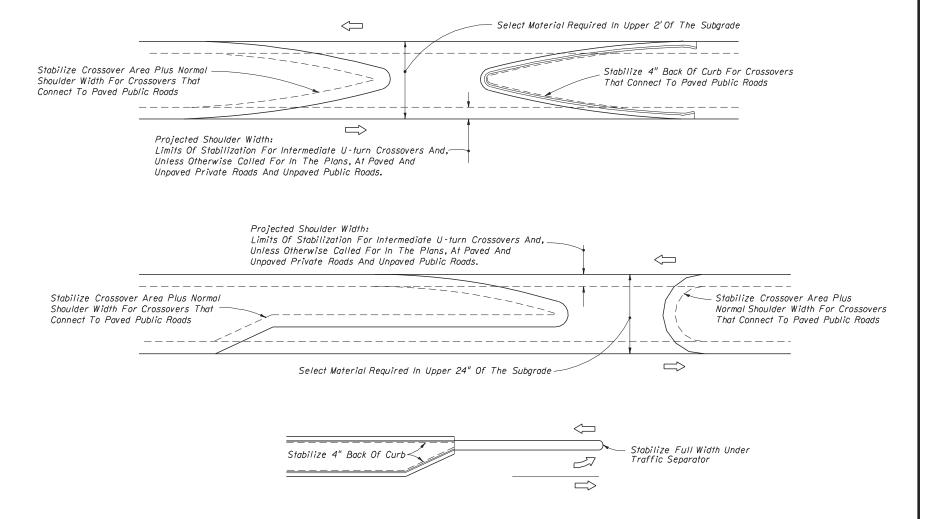
	Names	Dates	Approve	. //	e 11
Designed By	HMD	09/93		State Geotechi	0
Drawn By	HSD	09/93	Revision	Sheet No.	Index No.
Checked By	BTD	09/93	00	3 of 3	505



NOTES

- I. All material in the shaded area is excess base to be removed.
- 2. The cost for removal of excess base material shall be included in the contract unit price for base.
- 3. Payment for base shall be calculated using normal width.

REMOVAL OF EXCESS BASE MATERIAL



NOTES

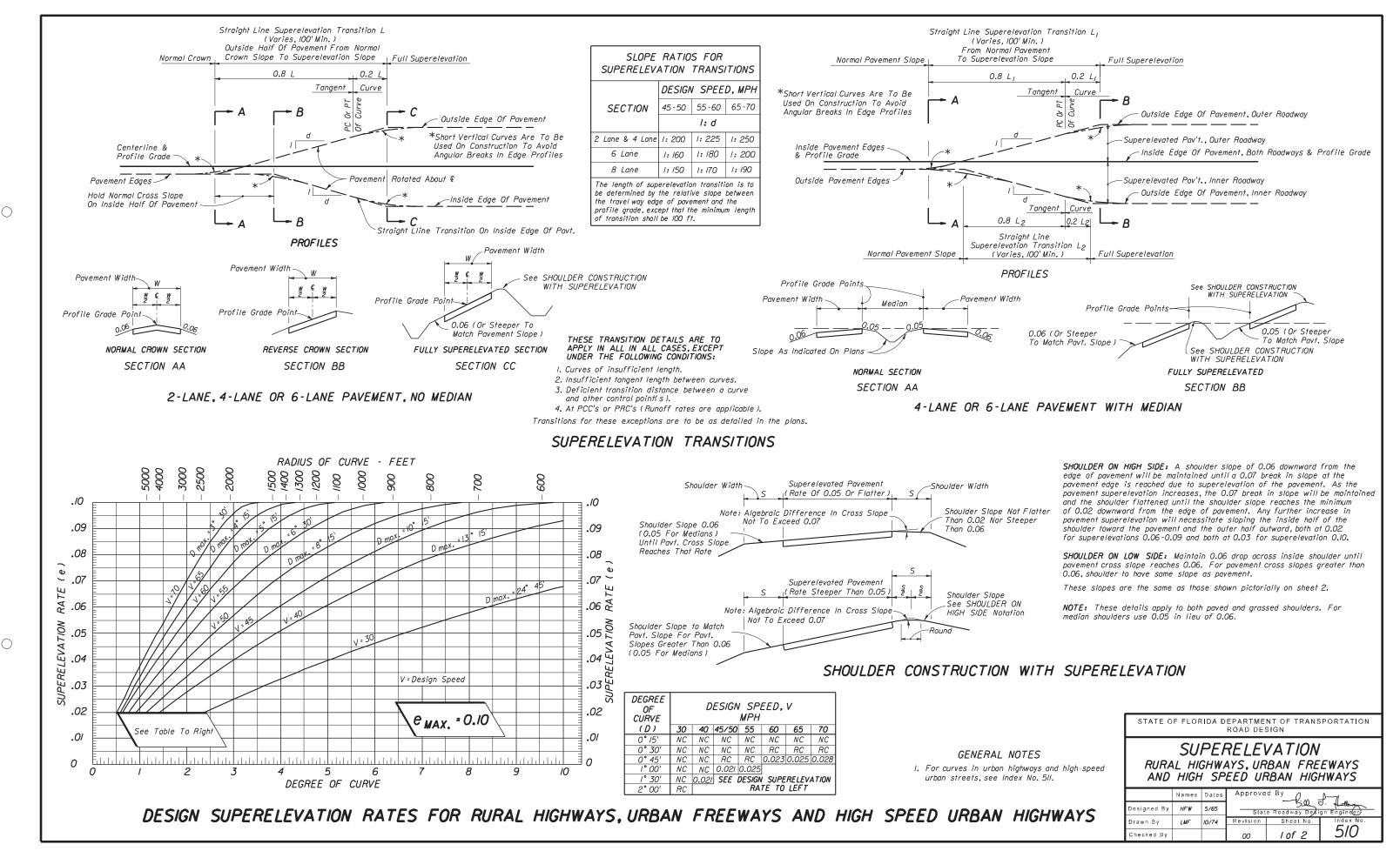
- I. When the median has curb or curb and gutter, stabilize 4" back of curb.
- 2. When the median has shoulder with no curb or curb and gutter, stabilize to normal shoulder width.
- 3. See the details above for stabilizing requirements at crossroads.
- 4. Stabilize entire area under all paved traffic islands.
- 5. Stabilize full width under all traffic separators.
- 6. Select material as defined on Index No. 505. For minor collectors and local facilities the depth of select material thickness may be reduced from 24" to 18".

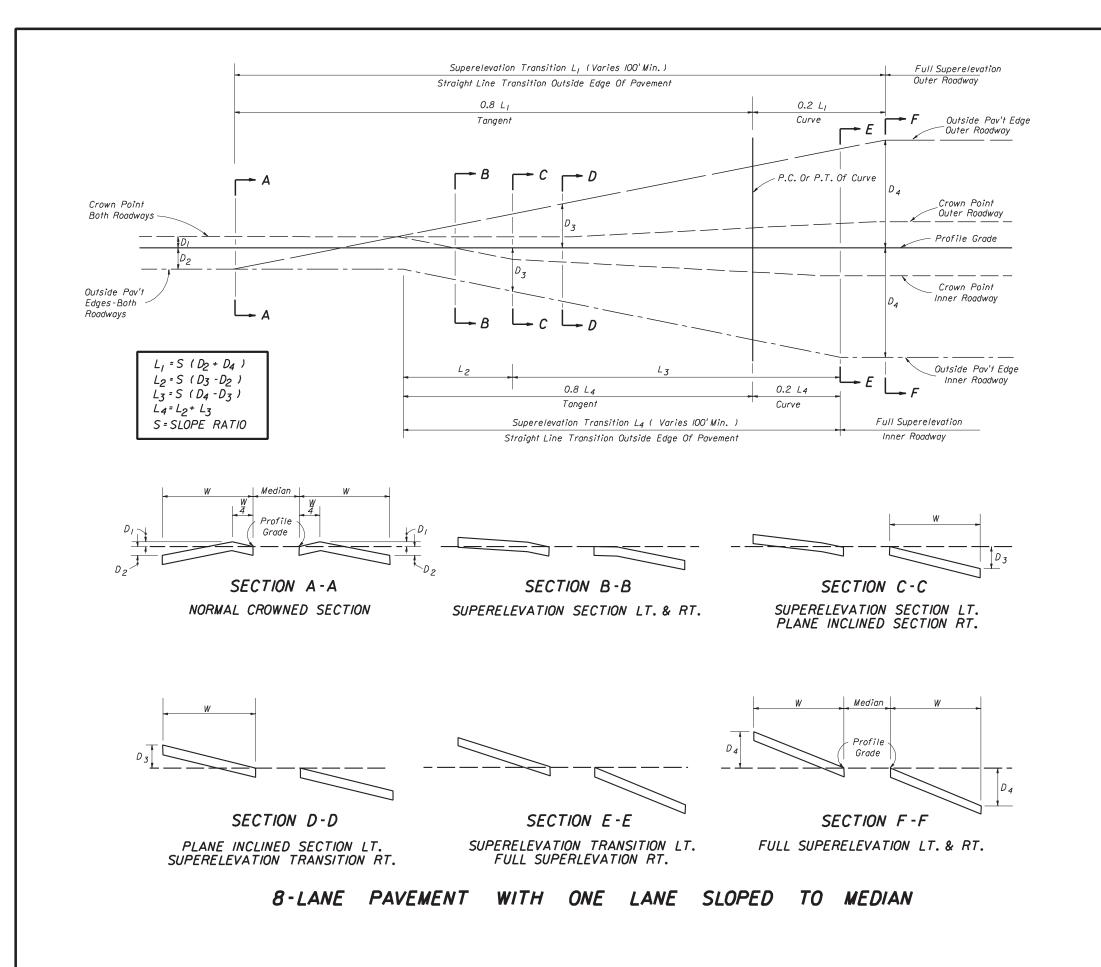
MEDIAN STABILIZING DETAILS

MISCELLANEOUS EARTHWORK DETAILS

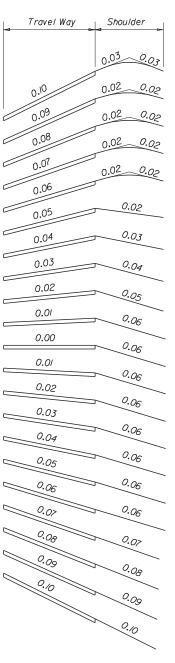
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

	Names	Dates	Approve	d By	P-71
Designed By	RL/WNL	65, 91		State Roadway	Design Engineer
Drawn By	HKE	05/91	Revision	Sheet No.	Index No.
Checked By	JVG/WNL	05/91	00	l of l	506





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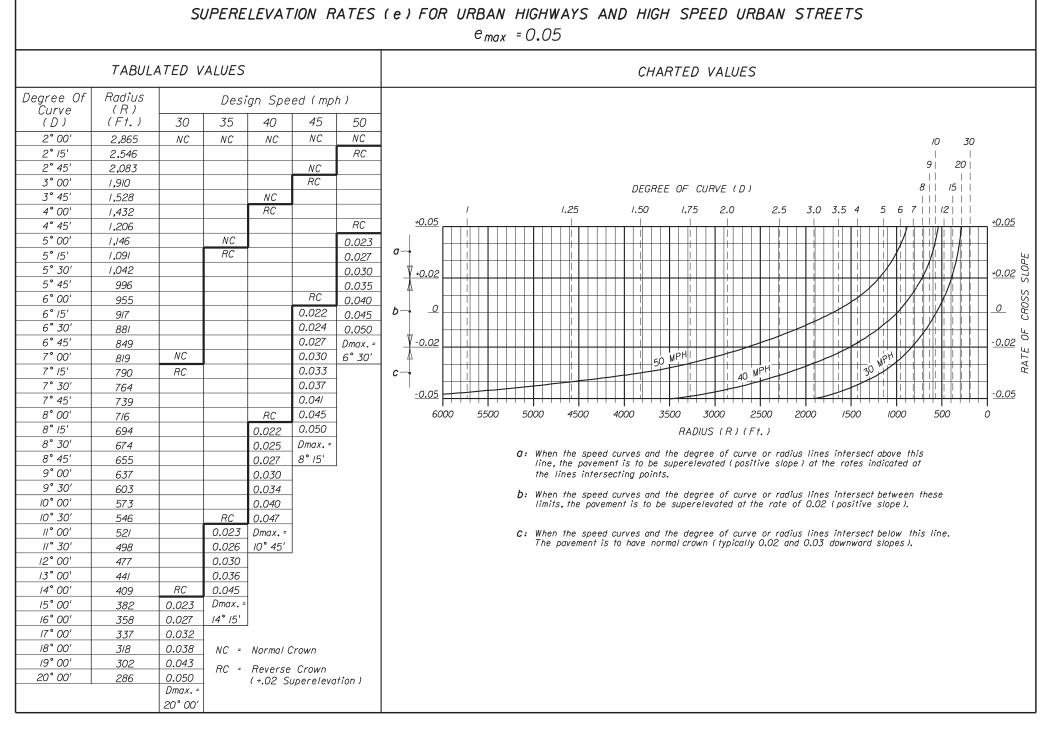
SLOPES OF TRAVELED WAY AND ABUTTING SHOULDERS

SHOULDER SLOPES ON SUPERELEVATION SECTIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SUPERELEVATION RURAL HIGHWAYS, URBAN FREEWAYS AND HIGH SPEED URBAN HIGHWAYS

	Names	Dates	Approve	d By	⊅ 1		
Designed By	WAL	8/77	State Roadway Design Engineer				
Drawn By	LMF	8/77	Revision	Sheet No.	Index No.		
Checked By	WAL	\$/77	00	2 of 2	510		



GENERAL NOTES

- Maximum rate of superelevation for urban highways and high speed urban streets shall be 0.05.
- Superelevation shall be obtained by rotating the plane successively about the break
 points of the section until the plane has attained a slope equal to that required by
 the chart. Should the rotation traverse the entire section and further superelevation
 be required, the remaining rotation of the plane shall be about the low edge of the
 inside travel lane.

Crown is to be removed in the auxiliary lane to the outside of the curve only when the adjoining travel lanes require positive superelevation.

- 3. When positive superelevation is required, the slope of the gutter on the high side shall be a continuation of the slope of the superelevated pavement.
- 4. In construction, short vertical curves shall be placed at all angular profile breaks within the limits of the superelevation transition.
- 5. The variable superelevation transition length "L" shall have a minimum value of 50 feet for design speeds under 40 MPH and 75 feet for design speeds of 40 MPH or greater.
- 6. Roadway sections having lane arrangements different from those shown, but composed of a series of planes, shall be superelevated in a similar manner.
- 7. For superelevation of lower speed urban streets, see the FDOT 'Manual Of Uniform Minimum Standards For Design, Construction And Maintenance For Streets And Highways'. For superelevation of curves on rural highways, urban freeways and high speed urban highways, see Index No. 510.

 $e_{max} = 0.05$

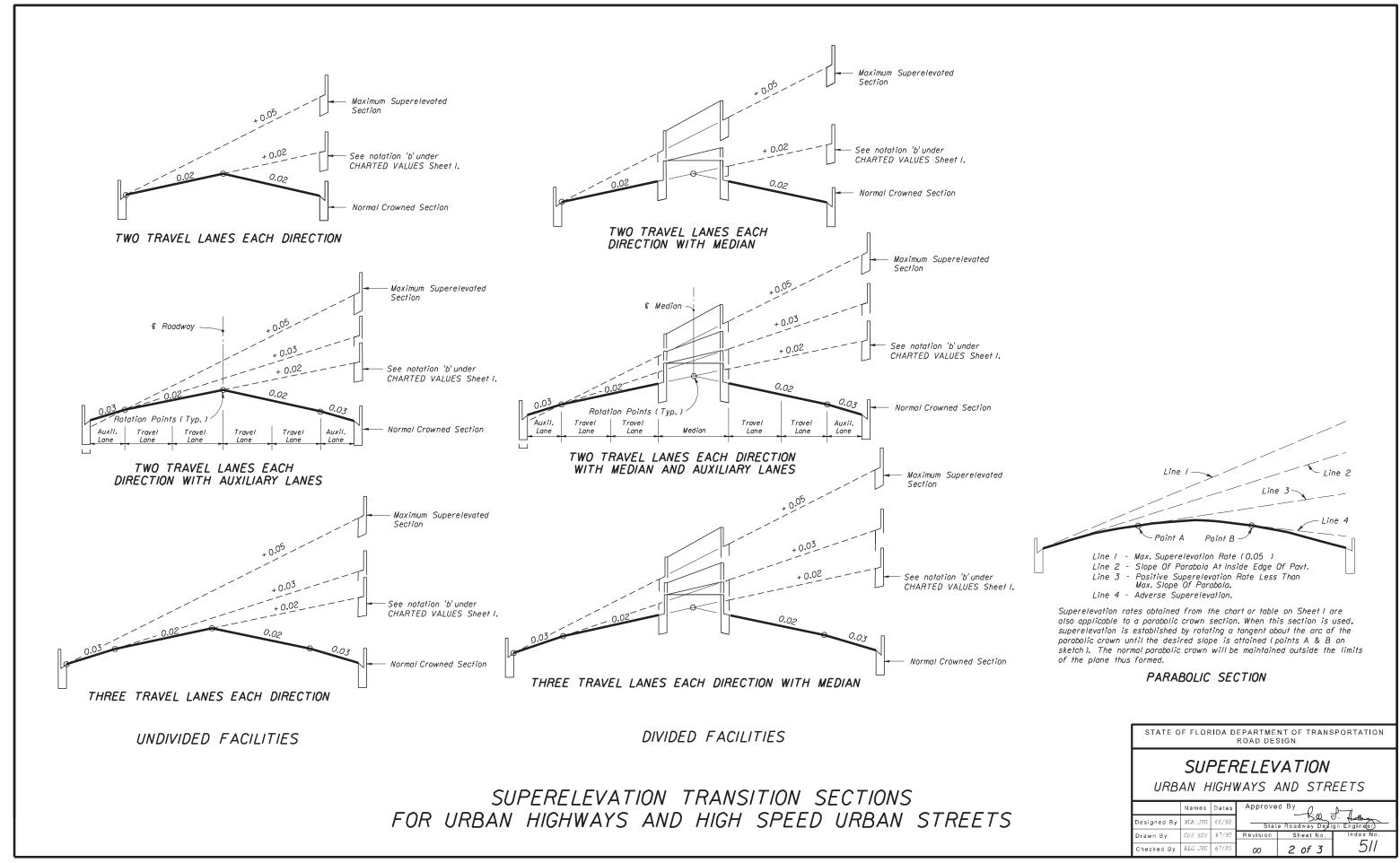
SUPERELEVATION FOR URBAN HIGHWAYS AND HIGH SPEED URBAN STREETS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

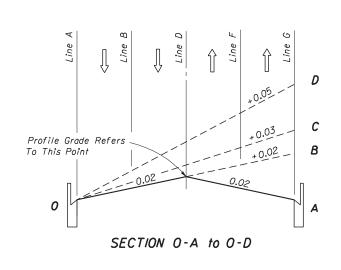
SUPERELEVATION

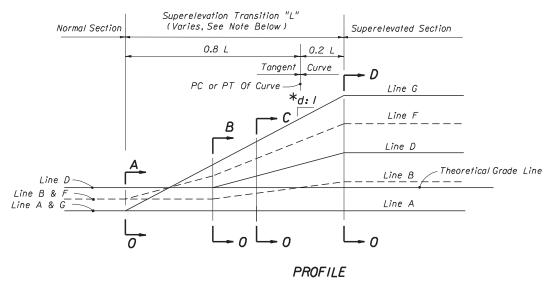
URBAN HIGHWAYS AND STREETS

	Names	Dates	Approve	d By	<u></u>
Designed By	WLB/JVG	66 & 90	Stat	e Roadway Degli	gn Engine(er)
Drawn By	CDR/HSD	67 & 90	Revision	Sheet No.	Index No.
Checked By	RLO/JVG	67 & 90	00	1 of 3	5//



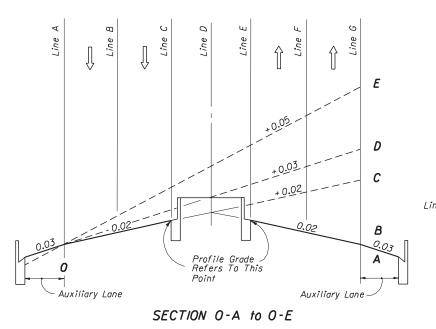
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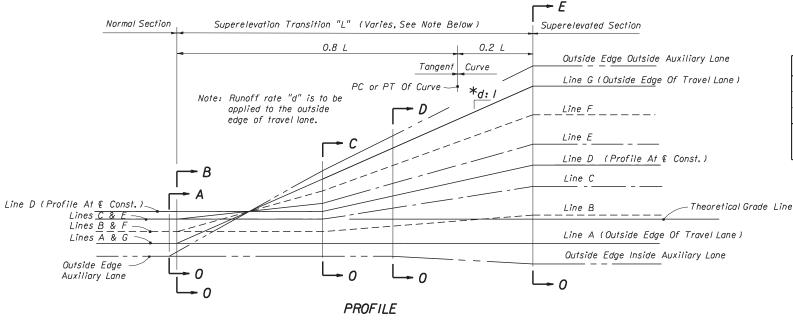




TWO LANES EACH DIRECTION

INE	DESCRIPTION						
Α	Inside Travel Lane						
В	Inside Lane Line						
С	Inside Median Edge Pavement						
D	€ Construction						
Ε	Outside Median Edge Pavement						
F	Outside Lane Line						
G	Outside Travel Lane						
ide A	nd Outside Are Relative To Curve Center						
	A B C D E F G						





*d (Slope Ratio)						
30 MPH	1: 100					
40 MPH	I: 125					
45-50 MPH△	/ : /50					
_						

△ 1: 125 May Be Used For 45 MPH Under Restricted Conditions.

TWO LANES EACH DIRECTION WITH MEDIAN AND AUXILIARY LANE

Note: The sections and profiles shown are examples of superelevation transitions. Similar schemes should be used for roadways having other sections.

EXAMPLE SUPERELEVATION SECTIONS AND PROFILES FOR URBAN HIGHWAYS AND HIGH SPEED URBAN STREETS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

SUPERELEVATION URBAN HIGHWAYS AND STREETS

	Names	Dates	Approve	d By	1-71	
Designed By	WLB/JVG	66/90	State Roadway Design Engineer			
Drawn By	CDR HSD	67/90	Revision	Sheet No.	Index No.	
Checked By	RLO, JVG	67/90	00	3 of 3	511	

	BASE THICKNESS AND OPTION CODES									
			Base Options							
Base Group	Structural Range	Base Group Pay Item Number	Limerock LBR 100	Cemented Coquina LBR 100	Shell Rock LBR 100	Bank Run Shell LBR 100	Graded Aggregate Base LBR 100	Type B-12.5	B-12.5 And 4" Granular Subbase, LBR 100 *	RAP Base
аѕе	truc	7Se		I			ber (Pe	T	I	I
B	S	Ğ	(.18)	(.18)	(.18)	(.18)	(.15)		(.30 & ./5)	
/	.6575	701	4"	4"	4"	4"	4 <u>/</u> "	^Δ 4"		5"
2	.8090	702	5"	5"	5"	5"	5 <u>/</u> "	^Δ 4"		
3	.95 -1.05	703	5 <u>/</u> "	5 <u>/</u> "	5 <u>/</u> "	5 <u>/</u> "	6 <u>/</u> "	^Δ 4"		
4	1.05 -1.15	704	6"	6"	6"	6"	7 <u>/</u> "	^Δ 4"		
5	1.25-1.35	705	7"	7"	7"	7"	8 <u>/</u> "	4½"		
6	1.35 -1.50	706	8"	8"	8"	8"	9"	5"		
7	1.50-1.65	707	8 <u>/</u> "	8 <u>/</u> "	8 <u>/</u> "	8 <u>/</u> "	10"	5 <u>/</u> "		
8	1.65-1.75	708	9 <u>/</u> "	9 <u>1</u> "	9 <u>/</u> "	9 <u>/</u> "	//"	5 <u>/</u> "		
9	1.75-1.85	709	10"	10"	10"	10"	12"	6"	4"	
10	1.90 - 2.00	710	//"	//"	//"	//"	ø _{13"}	6 <u>/</u> "	4 <u>1</u> "	
//	2.05 - 2.15	711	12"	12"	12"	12"	Ø _{14"}	7"	5"	
12	2.20-2.30	712	12 <u>1</u> "	12½"	12 <u>1</u> "	12½"		7 <u>/</u> "	5 <u>/</u> "	
13	2.35-2.45	713	Ø _{13<u>†</u>"}		Ø ₁₃ ½"	Ø _{13½"}		8"	6"	
14	2.45 - 2.55	714	Ø _{14"}	0 14"	0 14"	0 14"		8 <u>1</u> "	6 <u>/</u> "	
15	2.60 - 2.70	715						9"	7"	

GENERAL NOTES

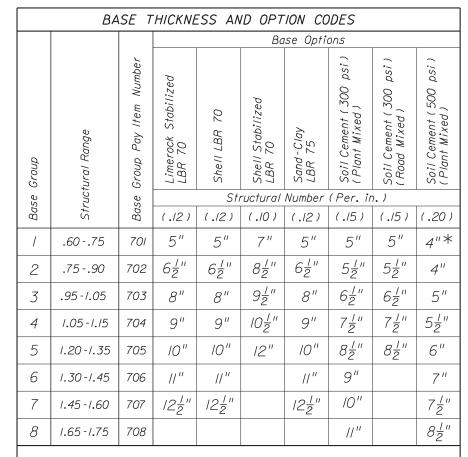
- I. On new construction and complete reconstruction projects where an entirely new base is to be built, the design engineer may specify just the Base Group and any of the unrestricted General Use Optional Bases shown in that base group may be used. Note, however, that some thick granular bases are limited to widening which prevents their general use.
- 2. Where base options are specified in the plans, only those options may be bid and used.
- 3. The designer may require the use of a single base option, for instance Type B-12.5 in a high water condition. This will still be bid as Optional Base.

- * For granular subbase, the construction of both the subbase and Type B-I2.5 will be paid for under the contract unit price for Optional Base. Granular subbases include Limerock, Cemented Coquina, Shell Rock, Bank Run Shell and Graded Aggregate Base at LBR 100. The base thickness shown is Type B-I2.5. All subbase thicknesses are 4".
- \emptyset To be used for widening only, three feet or less.
- △ Based on minimum practical thicknesses.
- ☐ Restricted to non-limited access shoulder base construction.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

OPTIONAL BASE GROUP AND STRUCTURAL NUMBERS

	Names	Dates	Approve	d By	Ø
Designed By	HMD	12/93		State Pavement	Design Engineer
Drawn By	HKH	12/93	Revision	Sheet No.	Index No.
Checked By	BTD	12/93	00	1 of 2	5/4



Not Recommended For 20 Year Design Accumulated 18 kip Equivalent Single Axle (ESAL) Loads Greater Than 1,000,000

Note

These base materials may be used on FDOT projects when approved in writing by the District Materials Engineer and shown in the plans.

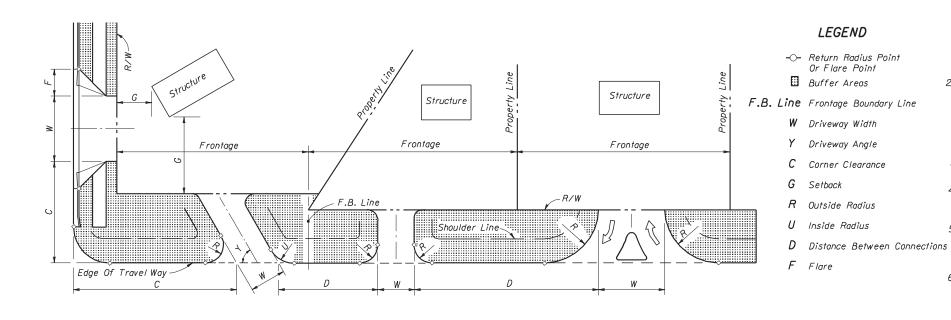
* Based On Minimum Practical Thickness

LIMITED USE OPTIONAL BASE GROUPS AND STRUCTURAL NUMBERS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

OPTIONAL BASE GROUP AND STRUCTURAL NUMBERS

	Names	Dates	Approve	' /)	7.4.4
Designed By	HMD	12/93			Design Engineer
Drawn By	HKH	12/93	Revision	Sheet No.	Index No.
Checked By	BTD	12/93	00	2 of 2	<i>514</i>



For Additional Information Refer To FDOT Rules Chapters 14-96 And 14-97.

SKETCH ILLUSTRATING DEFINITIONS

	UR	BAN (CURB & GU	TTER)	RURAL			
ELEMENT DESCRIPTION	I-20 Trips/Day or I-5 Trips/Hour	21-600 Trips/Day or 6-60 Trips/Hour 2-Way □	601-4000 Trips/Day 61-400 Trips/Hour 2-Way	I-20 Trips/Day or I-5 Trips/Hour	21-600 Trips/Day or 6-60 Trips/Hour 2-Way □	601-4000 Trips/Day or 61-400 Trips/Hour 2-Way □	
CONNECTION WIDTH W	12' Min. 24' Max.	24' Min. 36' Max.	24' Min. 36' Max.	12' Min. 24' Max.	24' Min. 36' Max.	24' Min. 36' Max.	
FLARE (Drop Curb) F	10' Min.	IO' Min.	N/A	N/A	N/A	N/A	
RETURNS (Radius) R & U	N/A	Δ	25' Min. 50' Std. 75' Max.	15' Min. 25' Std. 50' Max.	25' Min. 50' Std. 75' Max.	25' Min. 50' Std. (Or 3-Centered Curves)	
ANGLE OF DRIVE Y		60°-90°	60°-90°		60°-90°	60°-90°	
DIVISIONAL ISLAND (Throat Median)		4'-22' Wide	4'-22' Wide		4'-22' Wide	4'-22' Wide	
SETBACK G	I2' Min., All categories. See General Note No. 5.						

- Street or road intersection design, with possible auxiliary lanes and channelization, may be necessary. Intersection design, with possible auxiliary lanes and channelization, should be considered for connections with more than 4000 trips/days.
- □ "2-Way" refers to one "in" movement and one "out" movement i.e. not exclusive left or right turn lanes on the connection.
- \triangle Small radii may be used in lieu of flares as approved by the Department.

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DESIGN NOTE: I-Way connections will be designed to effectively eliminate unpermitted movements.

NOT INTENDED FOR FULL INTERSECTION DESIGN

SUMMARY OF GEOMETRIC REQUIREMENTS FOR TURNOUTS

GENERAL NOTES

LEGEND

Or Flare Point

- I. For definitions and descriptions of access connection "Categories" and access "Classifications" of highway segments, and for other detailed information on access to the State Highway System, refer to FDOT Rule Chapter 14-96, "State Highway Connection Permits Administrative Process" and Rule Chapter 14-97, "State Highway System Access Management Classification System And Standards"
- 2. For this index the term 'turnout' applies to that portion of driveways, roads or streets adjoining the outer roadway. For this index the term 'connection' encompasses a driveway, street or road and their appurtenant islands, separators, transition tapers, auxiliary lanes, travelway flares, drainage pipes and structures, crossovers, sidewalks, curb cut ramps, signing, pavement marking, required signalization, maintenance of traffic or other means of access to or from controlled access facilities. The turnout requirements set forth in this index do not provide complete intersection design, construction or maintenance requirements.
- 3. The location, positioning, orientation, spacing and number of connections and median openings shall be in conformance with FDOT Rule Chapter 14-97.
- 4. On Department construction projects all driveways not shown on the plans are to be reconstructed at their existing location in conformance to these standards, or, in conformance to permits issued during the construction project.
- 5. Driveways shall have sufficient length and size for all vehicular queueing, stacking, maneuvering, standing and parking to be carried out completely beyond the right of way line. Except for vehicles stopping to enter the highway, the turnout areas and drives within the right of way shall be used only for moving vehicles entering or leaving the highway.
- 6. Connections with expected daily traffic over 4000 vpd are to be constructed as intersecting streets or roads. The design requirement of this index and that of the local government will be used to select appropriate connection widths, radii and intersection design, subject to the approval of the Department.

For connections with expected daily traffic less than 4000 vpd, the Department will determine if drop curbs or radius returns are required in accordance with existing or planned connections. Where radius returns apply, the design requirements of this index and that of the local government will be used to select appropriate connection widths, radii and intersection design, subject to the approval of the Department.

For connections that are intended to daily accommodate either multi-unit vehicles or single unit vehicles exceeding 30' in length, returns with 50' radii are to be used, unless otherwise called for in the plans or otherwise stipulated by permit. Where large numbers of multi-unit vehicles will use the connection, the connection width and radii are to be increased and auxiliary lanes, tapers, lane flares, separators and/or islands constructed, as determined by the Department to be necessary for safe turning movements.

- 7. Any connection on a highway having a posted or operating speed over 45 mph shall have radial returns. Any connection requiring or having a specified median opening with left turn storage and served directly by that opening shall have radial returns.
- 8. Where a connection is intended to align with a connection across the highway, the through lanes are to align directly with the corresponding through lanes.
- 9. For new connections and for connections on all new construction and reconstruction projects, pavement materials and thicknesses shall meet the requirements applicable to either that detailed for "Urban Flared Turnouts", or, that described in "Table 5/5-1" for connections with radial returns and/or auxiliary lanes.
- 10. The responsibility for the cost of construction or alteration to an access connection shall be in accordance with FDOT Rule Chapter 14-96.

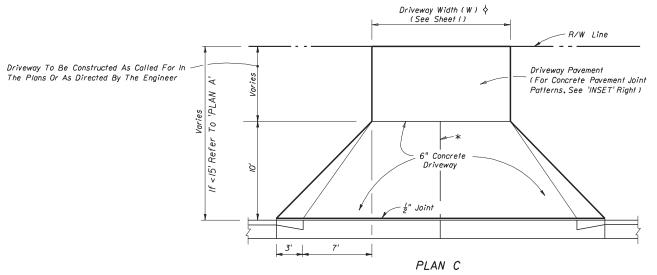
DESIGN NOTES

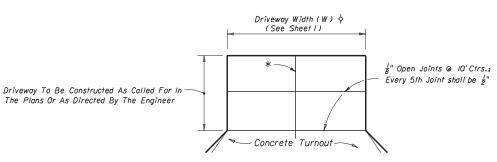
I. Prior to the adoption of FDOT Rules Chapters I4-96 and I4-97, connections to the State Highway System were defined and permitted by Classes. Connections have been redfined by Categories under Rule 14-96; and, the term "Class" has been applied to highway segments of the State Highway System as defined under Rule 14-97.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

TURNOUTS

	Names	Dates	Approve	d By	1
Designed By	COMV/JT	90/91		State Roadway	Design Engineer
Drawn By	HSD	03/91	Revision	Sheet No.	Index No.
Checked By	JVG	03/91	00	I of 6	5/5

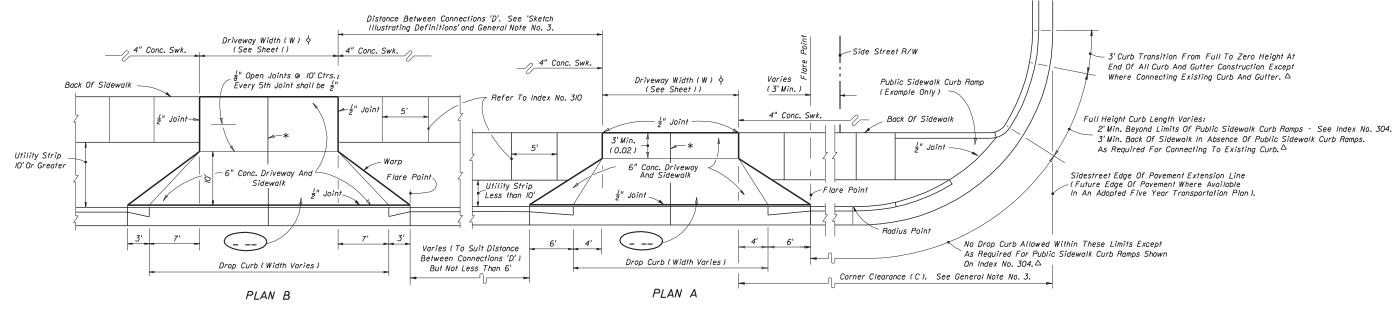




JOINT PATTERN WHEN CONCRETE DRIVE CONSTRUCTED INSET

TURNOUT WITHOUT SIDEWALK

SPECIAL NOTES FOR URBAN FLARED TURNOUTS



TURNOUT WITH SIDEWALK AND UTILITY STRIP (10' OR GREATER)

TURNOUT WITH SIDEWALK AND UTILITY STRIP (LESS THAN 10')

Footnotes:

All $\frac{1}{2}$ " joints shall be constructed with preformed joint filler.

- * $\frac{1}{8}$ " Open joints placed at equal (20' max.) intervals for driveways over 20' wide. Joints in curb and gutter to match joints in driveways.
- △ When connecting to sidestreet curb and gutter sections. the no drop curb limits should extend back to the sidestreet radius point. With or without curb and gutter, no driveway should encroach on the corner radius.
- ♦ Driveways (6" concrete) shall be of a uniform width (W) to the right of way line.



Alpha-numeric identification of a flared driveway type specifically called for in the plans, see sheets 3 and 4.

- I. Driveway 6" concrete payement and drop curb shall meet the material and construction requirements of Sections 522 and 520 respectively of the FDOT Standard Specifications. The driveway foundation shall meet the requirement of Subarticle 522-4.
- 2. For details of drop curb and public sidewalk curb ramps refer to Indexes Nos. 300 and 304 respectively.
- 3. Where turnouts are constructed within existing curb and gutter, the existing curb and gutter shall be removed either to the nearest joint beyond the flare point or to the extent that no remaining section is less than 5' long; and, drop curb constructed in accordance with Notes
- 4. Cost for preformed joint filler shall be included in the cost for the concrete payement (concrete sidewalk. 6" thick).
- 5. For turnouts with radial returns see the requirements under the "Summary Of Geometric Requirements For Turnouts", the "General Notes", the details of "Rural Turnout Construction" and the detail of "Limits Of Clearing & Grubbing, Stabilization And Base At Intersections".

- 6. Department maintenance of payement shall extend out to the right of way or 2' back of sidewalk, whichever distance is less.
- 7. The maintenance and operation of highway lighting, traffic signals, associated equipment, and other necessary devices shall be the responsibility of a public agency.
- 8. All pavement markings on the State highways, including acceleration and deceleration lane markings, and signing installed for the operation of the State highway shall be maintained by the Department.
- 9. All signing and marking installed for the operation of the connection (such as stop bars and stop signs for the connection) shall be the responsibility of the permittee.
- IO. Turnouts will be paid for under the contract unit price for Concrete Sidewalk (6" Thick), SY.

DESIGN NOTES FOR URBAN FLARED TURNOUTS

slopes that can cause overhang drag for representative standard passenger vehicles under fully loaded conditions; or, those with slopes that can cause drivers who are leaving the roadway to slow or pause to the extent that traffic demand volumes will be impeded.

Driveways indicated as 'Marginal Applications' are those with slopes that can cause overhang drag for representative standard passenger vehicles under fully loaded conditions when the driveway is located on the low side of fully superelevated

Driveways indicated as 'General Applications' are those with slapes that can readily accommodate representative standard passenger vehicles and those and those that can accommodate representitive standard trucks, vans, buses and recreational vehicles operating under normal crown and superelevation

Note: See sheet I for 'GENERAL NOTES'

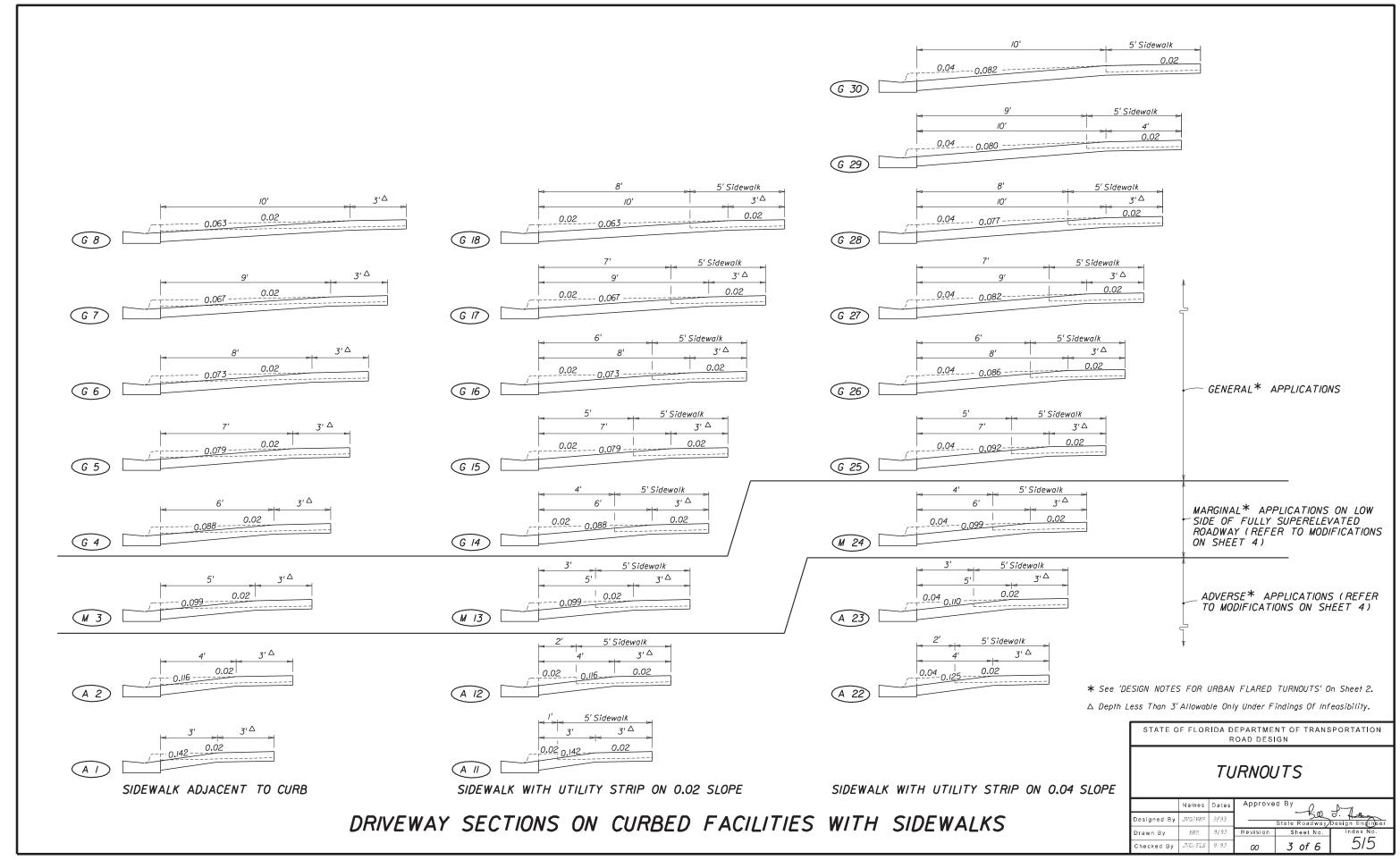
- I. Driveways indicated as 'Adverse Applications' are those with 2. The standard flared driveways on this index may not accommodate vehicles with low beds, low undercarriage or low appendage features. Where such vehicles are design vehicles driveways are to have site specific flare designs or Catagory III designs.
 - 3. When specific flare type driveways are to be constructed, the type shall be designated in the plans using the assigned alpha-numeric designation

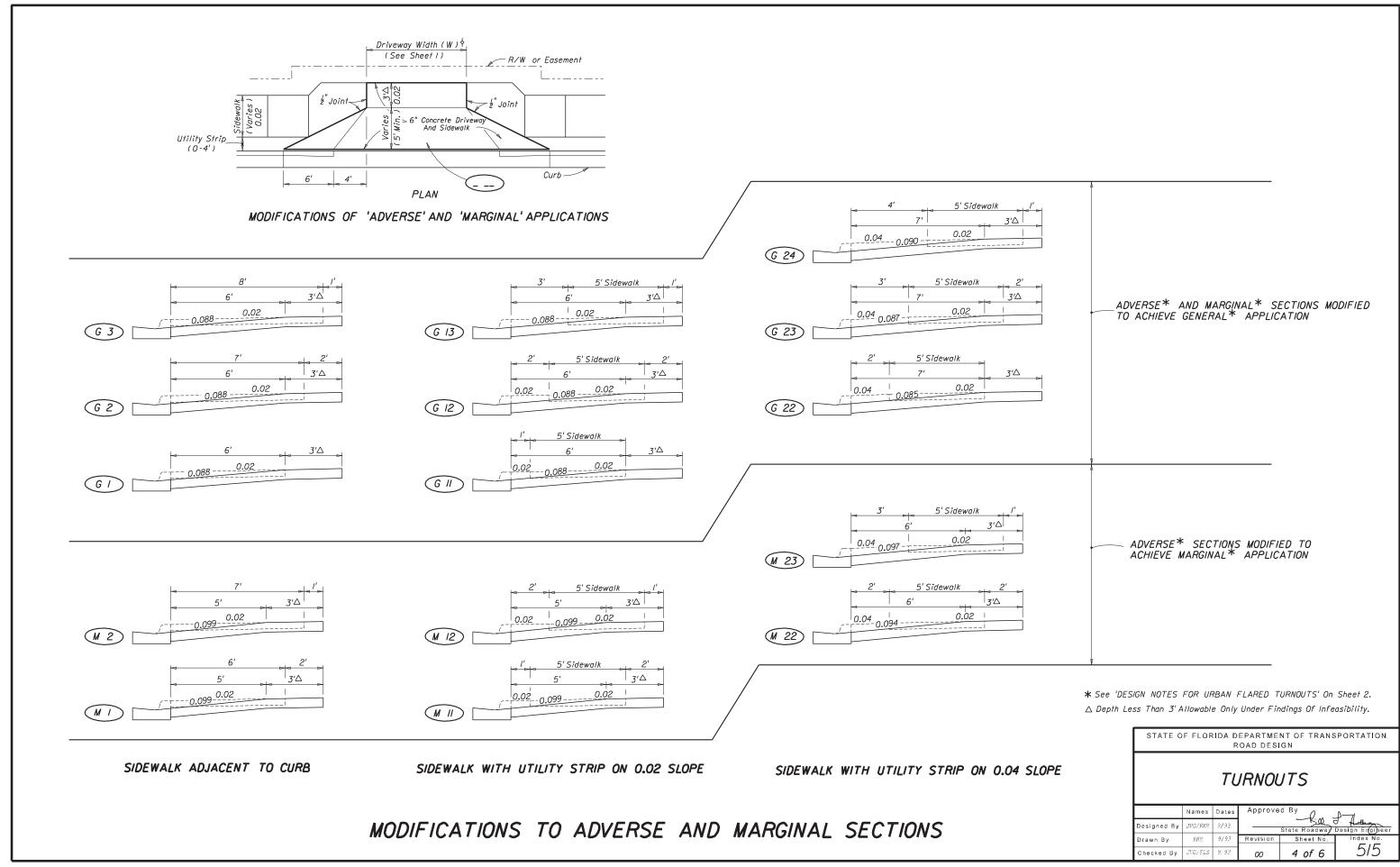
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

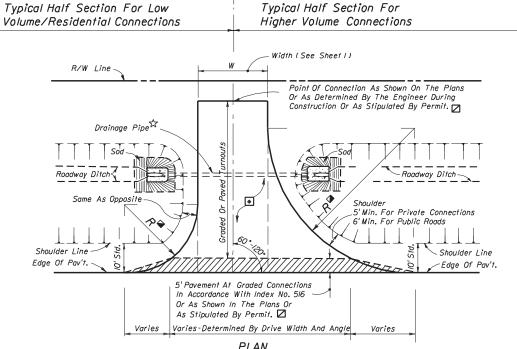
TURNOUTS

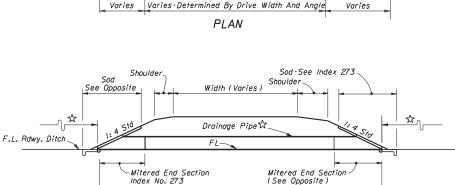
	Names	Dates	Approve	d By	A-71
Designed By	JVG/IIKII	09, 93		State Roadway	Design Engineer
Drawn By	HKH	09/93	Revision	Sheet No.	Index No.
Checked By	JVG	09/93	00	2 of 6	5/5

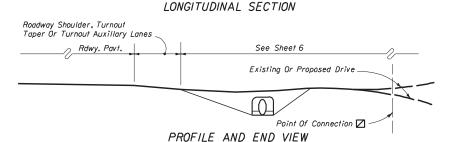
URBAN FLARED TURNOUTS











Varies-Determined By Slope Intercept

- Drainage pipe size and length shall be that shown on the plans, or as stipulated by permit, or, as determined by the Engineer during construction. The size shall be at least that established by the FDOT District, but not less than 15" diameter or equivalent. For minimum cover over drainage pipe see Index No. 205. Pipe arch or elliptical pipe may be required to obtain necessary cover. At minimal cover applications a modified povement apron is permitted. See 'PERMISSIBLE PAVEMENT MODIFICATION' Index No. 273. For spacing between adjacent pipe end treatments see Index No. 273.
- Stable material may be required for graded turnouts to private property as directed by the Engineer in accordance with Section 102-6 of the Standard Specifications.
- The 5' pavement at graded connections is not required where there is paved shoulder 4' or more in width. The 5' pavement requirement may be waived for connections serving one or two homes or field entrances with less than 20 trips per day, or 5 trips per hour as approved by permit or by the Engineer, or when not itemized in the plans.

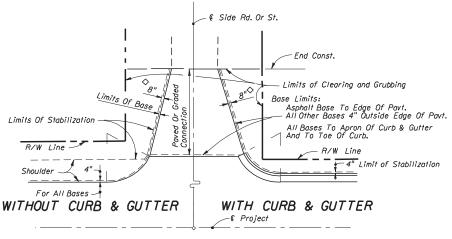
Paved turnouts are to be constructed for all paved connecting facilities. The connecting point will be determined by the Engineer.

Paved turnouts are to be constructed for all business, commercial, industrial or high volume residential graded connecting facilities. The connecting point shall be 30' from edge of roadway pavement or at R/W line, whichever is less.

Paved turnouts are to be constructed for all connecting facilities over 4000 vehicles per day. The connecting point shall be at the R/W line.

See "Summary Of Geometric Requirements For Turnouts" chart for return radii lengths and supplemental information.

RURAL TURNOUT CONSTRUCTION



♦ 8" Or Match Exist. Stabilizing (8" Min.)

LIMITS OF CLEARING & GRUBBING, STABILIZING AND BASE AT INTERSECTIONS

MATERIAL TYPES AND THICKNESSES IN DRIVING AREAS FOR RURAL AND URBAN CONNECTIONS

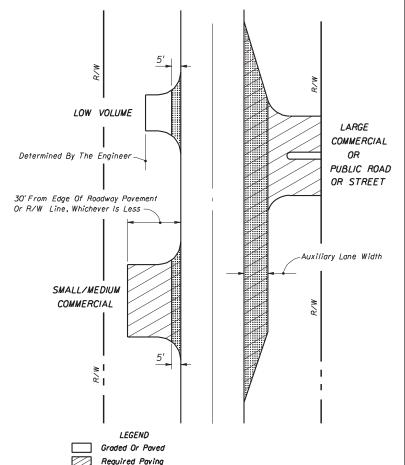
		Thickness (in.)		
Course	Malerials ^②	Connections 3	Roadway ⁴	
Structural	Ashpaltic Concrete	<i>1</i> "	/ <u>/</u> "	
Bases	Optional Base (See Index No. 514)	0.B.G. /	0.B.G. 3	

- (T) Minimum thickness.
- ② All materials shall be approved by the Department prior to being placed.
- 3 Connection structure other than traffic lanes. See Notes I and 2 below.
- Travel way flares (bypass lanes), auxiliary lanes serving more than a single connection, and all median crossovers including their auxiliary lanes and/or transition tapers. See Notes I and 2 below.

NOTES

- . The pavement should be structurally adequate to meet the expected traffic loads and should not be less than that shown above except as approved by the Department for graded connections. Other Department approved pavement equivalences may be used at the discretion of the Engineer. For additional information see Index No. 514.
- Auxiliary lanes and their transition tapers shall be the same structure as the abutting roadway pavement or any of the roadway structures tabulated above, whichever is thicker.
- 3. If an asphalt base course is used for a turnout, its thickness may be increased to match the edge of roadway pavement in lieu of a separate structural course. 6" of Portland cement concrete will be acceptable in lieu of the asphalt base and structural courses. See Notes 4 and 5 below.
- A structural course is required for flexible povements when they are used for auxiliary lanes serving more than a single connection.
- 5. Connections paved with Portland cement concrete shall be Class I concrete at least 6" thick. The Department may require greater thickness when called for in the plans or stipulated by permit. Materials and construction are to conform with FDOT Standard Specifications Sections 346, 350 and 522.
- 6. The Department may require other pavement criteria where local conditions warrant.

PAVEMENT STRUCTURE FOR TURNOUTS AND AUXILIARY LANES TABLE 515-1



Limits Of Department Maintenance

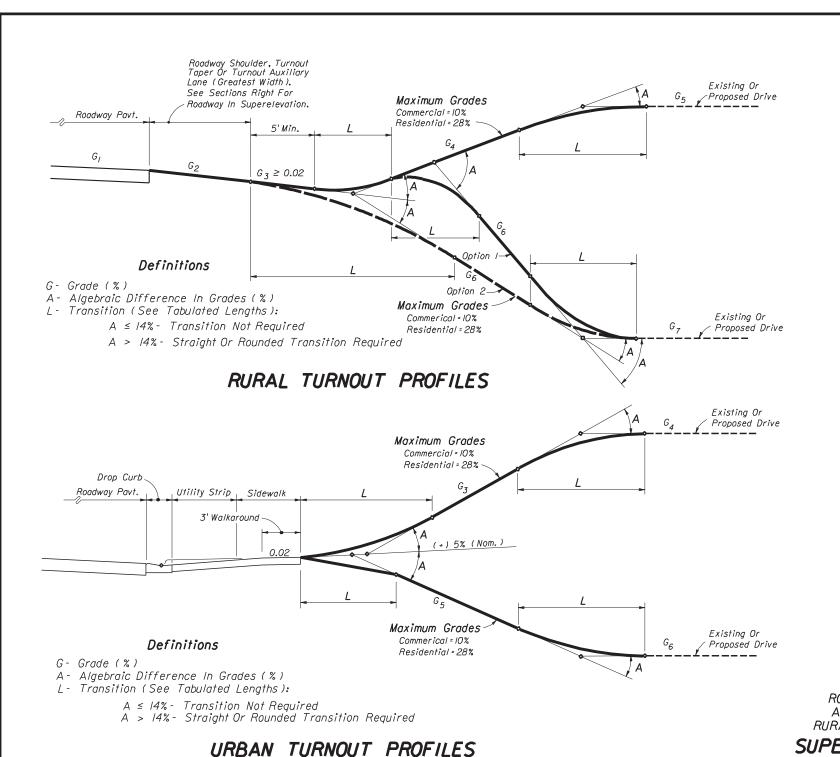
- NOTES

 I. Auxiliary lane pavements and crossover pavements shall be maintained by the Department
- 2. Department maintenance of turnout pavement shall extend out to 5' from edge of the travel way or limits of paved shoulders, and, extend to include auxiliary lanes. The remainder of any turnout paved area on the right of way shall be maintained by the owner or his authorized agent. As a function of routinely reworking shoulders, the Department may grade and shape existing material on non-paved areas beyond the maintained povement.
- 3. Control and maintenance of drainage facilities within the right of way shall be solely the responsibility of the Department, unless specified differently by Department permit
- The maintenance and operation of highway lighting, traffic signals, associated equipment, and other necessary devices shall be the responsibility of a public agency.
- All pavement markings on the State highways, including acceleration and deceleration lane markings, and signing installed for the operation of the State highway shall be maintained by the Department.
- All signing and marking installed for the operation of the connection (such as stop bars and stop signs for the connection) shall be the responsibility of the permittee.

LIMITS OF CONSTRUCTION AND MAINTENANCE FOR RURAL CONNECTIONS

00

5 of 6



 \bigcirc

G ₂ Slopes_	
(See Rural Turno Profile, Left)	ut _
Fromle, Lenn	
	0.03 0.03
G ₁ 0.10	
GI	0.02 0.02
G ₁ =0.09	0.02 0.02
G ₁ = 0.08	0.02 0.02
	0
G ₁ =0.07	0.02 0.02
G ₁ = 0.06	
G ₁ = 0.05	0.02
G ₁ = 0.04	0.03
G ₁ = 0.03	0.04
- 0.02	
G _I = 0.02	0.05
G _I = 0.01	0.06
G _I = 0.00	0.06
G ₁ = 0.01	0.06
G ₁ = 0.02	
G ₁ = 0.03	0.06
G1 = 0.04	0.06
G ₁ = 0.05	0.06
G ₁ = 0.06	0.06
G ₁ = 0.07	0.06
G ₁ = 0.08	0.07
61 - 0.09	
	0.08
9/ 50:10	0.09
	0.10
	,

Go Slones

RECOMMENDED TURNOUT PROFILE
TRANSITION LENGTHS (L) (FT)

LENGTHS (L) (FT.)

ROUNDED

Desirable Minimum

0

10

11

11.5

12

13

14

15

16

17

18

19

20

22

23

24

25

Rounded: Either circular, parabolic or spline curvature. The plans or the Engineer may

Desirable: Desirable minimum lengths.

Minimum: Absolute minimum lengths.

Greater lengths than minimum and desirable are recommended where practical for flatter and smoother profile.

12.5

10.5

10

10

10

10

10

11

13

15

15

16

17

22

23

24

26

27

29

30

32

33

34

36

37

specify a particular type of curvature.

SAGS

Desirable Minimum Desirable Minimum

10

10 10

10

10

12

13

14

16

16

17

18

18

19

20

21

22

23

25

26

28

29

3/

32

34

35

36

ROUNDED

0

10

11

12

13

/3.5

14.5

15

18

20

21

22

24

25

27

28

30

31

12.5

STRAIGHT

3

6

8

8.5

9.5

10

10.5

11.5

12

NA

9

11

12

/3

14

15

16

17

17

18

NA

NA

NA

NA

NA

NA

NA

NΑ

NA

NA

NA

NA

CRESTS

STRAIGHT

Pesirable | Minimum

2.5

3.5

4.5

5.5

7.5

8.5

NA

6-/3%

14%

15%

17%

18%

19%

20%

21%

22%

23%

24%

25%

26%

27%

28%

29%

30-31%

32-33%

34-36%

37 - 38%

39-41%

42 - 43%

44 - 46%

47 - 48%

49 - 51%

52-54%

55 - 56%

10

11

12

12

13

14

NA

ROADWAY PAVEMENT SLOPES AND SLOPES OF ABUTTING RURAL TURNOUT SURFACES (G₂)

SUPERELEVATION SECTIONS

PROFILE AND STORMWATER RUNOFF NOTES

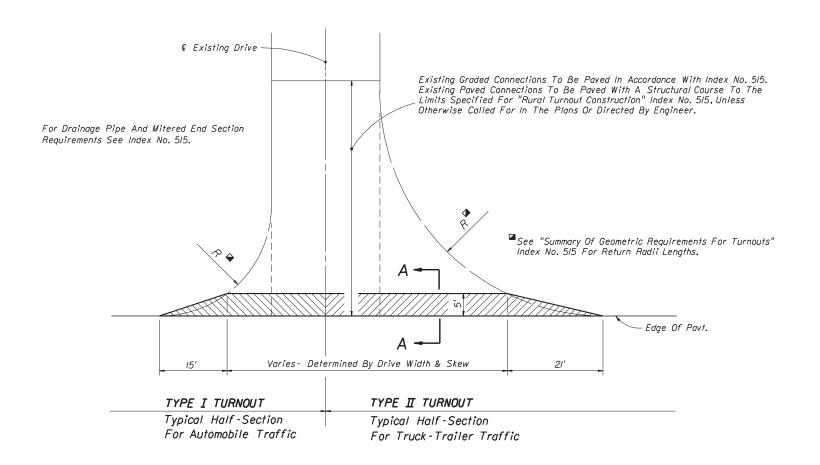
- I. Turnouts shall neither cause water to flow on or across the roadway pavement, nor cause water ponding or erosion within the State right of way. On all rural turnouts the transition (L) nearest the roadway shall be sloped or crowned to direct stormwater runoff to the roadside ditch. Inlets, flumes or other appropriate runoff control devices shall be constructed when runoff volumes are sufficient to cause erosion of the shoulder. Similar runoff control devices shall be constructed as necessary to properly direct and control the stormwater runoff on urban turnouts.
- The Option I profile is intended for locations where roadway, turnout taper and auxiliary lane stormwater runoff volumes are relatively large. The Option 2 profile is intended for locations where runoff volumes are relatively small and/or where there is no roadside ditch.

TURNOUT PROFILES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

TURNOUTS

	Names	Dates	Approve	d By	♪ ─// .
Designed By				State Roadway	Design Engineer
Drawn By	HSD	08/82	Revision	Sheet No.	Index No.
Checked By	JVG	08/82	00	6 of 6	5/5

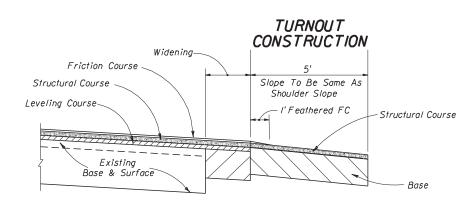


AREAS FOR ONE 5' DEEP TURNOUT (SY)								
	Intersection							
Drive Width	Nor	mal	Skewed					
(Ft.)	Type I	Туре Д	Type I	Type II				
12	26	51	3/	60				
14	27	52	33	61				
16	28	53	34	63				
18	29	54	35	64				
20	3/	55	37	65				
22	32	56	38	67				
24	33	57	39	68				
26	34	58	40	69				
28	35	59	42	70				
30	36	61	43	72				
32	37	62	44	73				
34	38	63	46	74				
36	39	64	47	76				
38	41	65	48	77				
40	42	66	49	78				
42	43	67	5/	79				
44	44	68	52	81				
46	45	69	53	82				
48	46	71	55	83				
50	47	72	56	85				
52	48	73	57	86				
5 <i>4</i>	49	74	58	87				
56	5/	75	60	88				
58	52	76	61	90				
60	53	77	62	91				

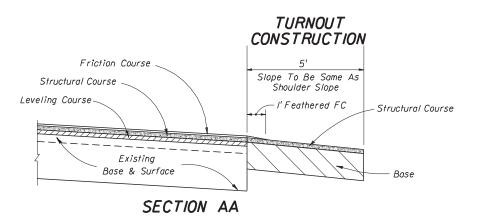
PAVEMENT STRUCTURE FOR 5'DEEP TURNOUTS						
Course	Material	Minimum Thickness				
Structural	Asphaltic Concrete	<i>I"</i>				
Base	Optional Base (See Index No. 5/4)	0.B.G. /				

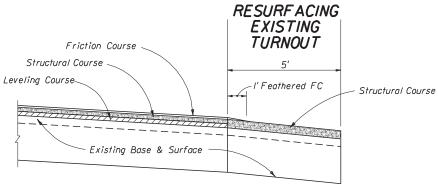
Notes:

- Turnout structural course to be the same material as roadway leveling or structure course. Structural course not required if asphalt base course and its thickness increased to match edge of roadway pavement.
- 2. Any Department approved pavement structure equivalence may be used at the discretion of the Engineer.
- 3. Additional structural strength may be required if heavy truck loads are anticipated.



SECTION AA WITH WIDENING





SECTION AA

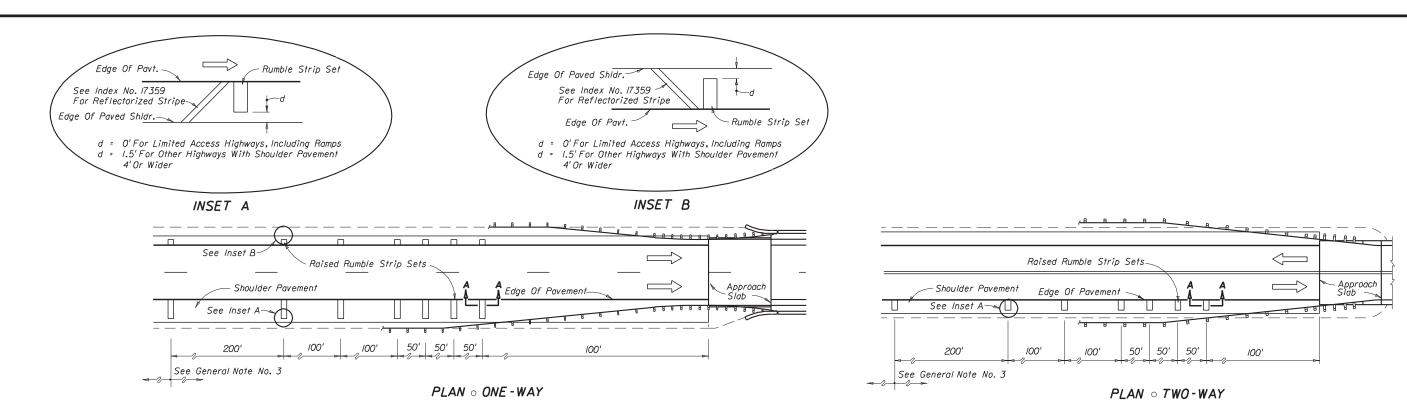
GENERAL NOTES

- Turnouts are to be constructed or resurfaced for low volume (single family, duplex, farm, etc.) residential connections as directed by the Engineer.
- 2. Turnout construction not required for low volume residential connections where roadway shoulders are paved.
- 3. Connections outside the 5' limit are to be constructed as directed by the Engineer.
- 4. The contract unit price for Turnout Construction includes the cost for excavation and base.
- 5. Payment for structural course to be included in roadway resurfacing pay item.
- 6. Payment for feathering friction course to be included in the unit price for Asphaltic Concrete Friciton Course placed on the roadway. Feathered areas will not be included in measured quantities. Feathering not required for FC-5 friction course.

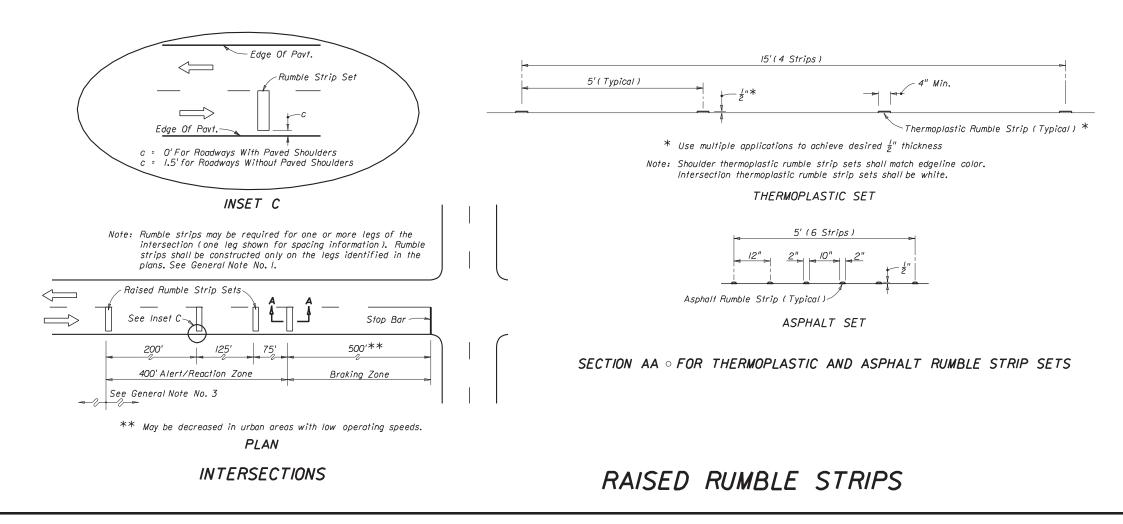
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TURNOUTS RESURFACING PROJECTS

	Names	Dates	Approve	d By	♪ 71
Designed By	DCB	11/77		State Roadway	Design Engineer
Drawn By	HKH	11/77	Revision	Sheet No.	Index No.
Checked By	JVG	11/77	00	l of l	5/6



STRUCTURES WITH LESS THAN FULL WIDTH SHOULDERS



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GENERAL NOTES FOR RAISED RUMBLE STRIPS

- I. Raised rumble strips shall be constructed on all paved shoulders approaching structures, where the structure shoulder width is less than the usable shoulder width of the approach roadway. Raised rumble strips at intersections shall be constructed only when specified in the plans.
- Raised rumble strips are to be constructed in accordance with Section 546 of the Specifications.
- 3. When any portion of a curve falls within the limit of rumble strips shown in these details, additional rumble strip sets spaced at 200' centers shall be constructed throughout the remainder of the approaching curve.

Checked By

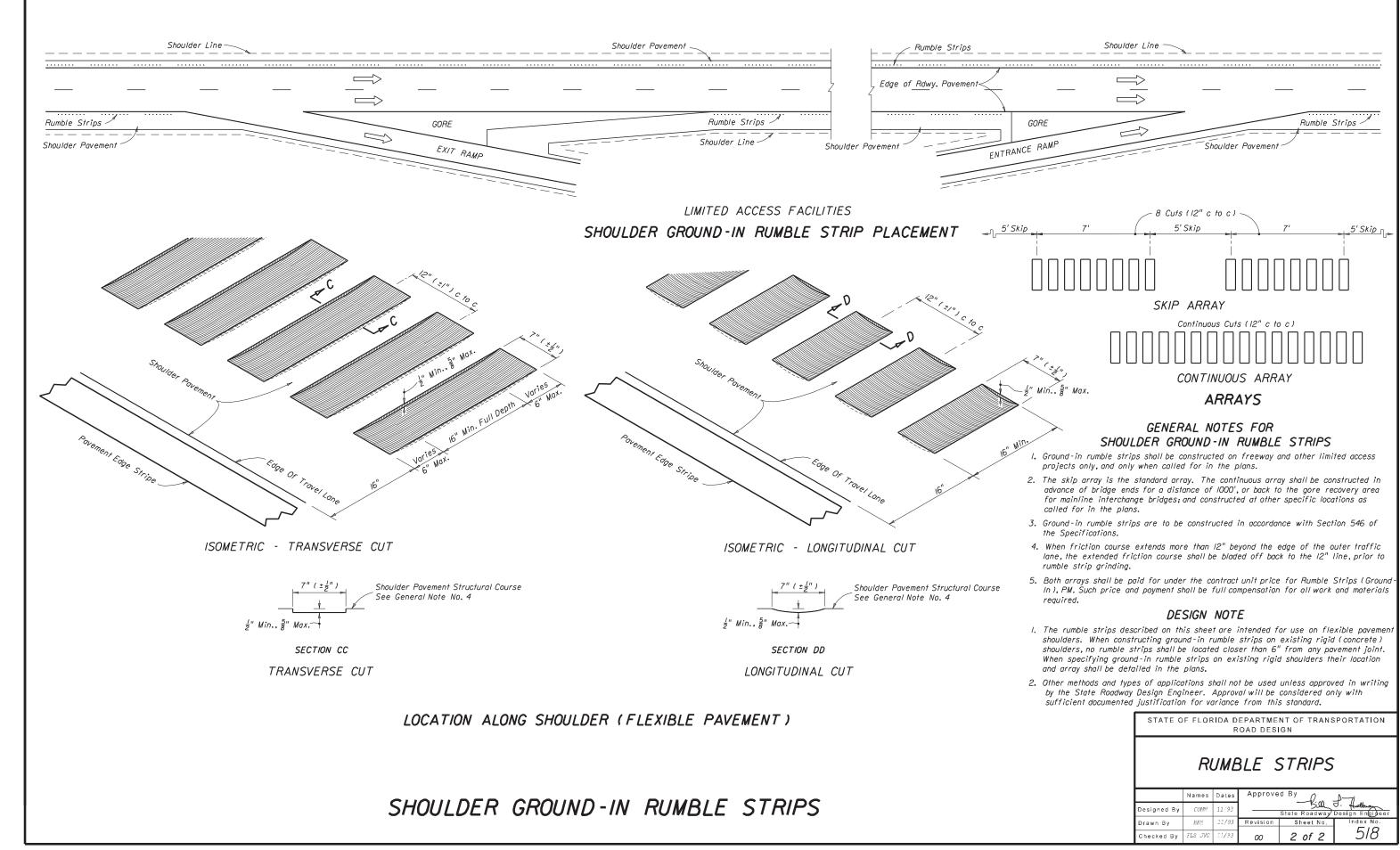
4. Raised rumble strips shall be paid for per set under the contract unit price for Rumble Strips Sets, PS. Such price and payment shall be full compensation for all work and materials required without adjustment due to width of payement receiving the strips or length of strips.

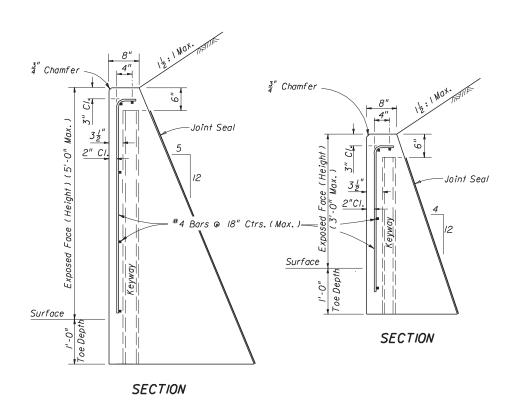
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RUMBLE STRIPS

Names Dates Approved By State Roadway Design Engineer

oo | 1 of 2 | .





ESTIMATED QUANTITIES FOR WALL

CLASS I CONCRETE (CY)

0.07

0.13

0.20 0.32 0.43

HEIGHT (EXPOSED FACED) PER LINEAR FOOT OF WALL

STEEL (LB)

 \bigcirc

 \bigcirc

Ŧ	Joint Seal
'ly Spaced	
() Ci B Pi	Nories 8" Varies Layers Of 55# Smooth Roofing All Contact Surfaces Of Ancrete And Roofing With Cut- ack Asphalt. Stop Roofing Apper 6" Below Top Of Wall.) EY DETAIL (TOP VIEW)

GRAVITY WALL NOTES

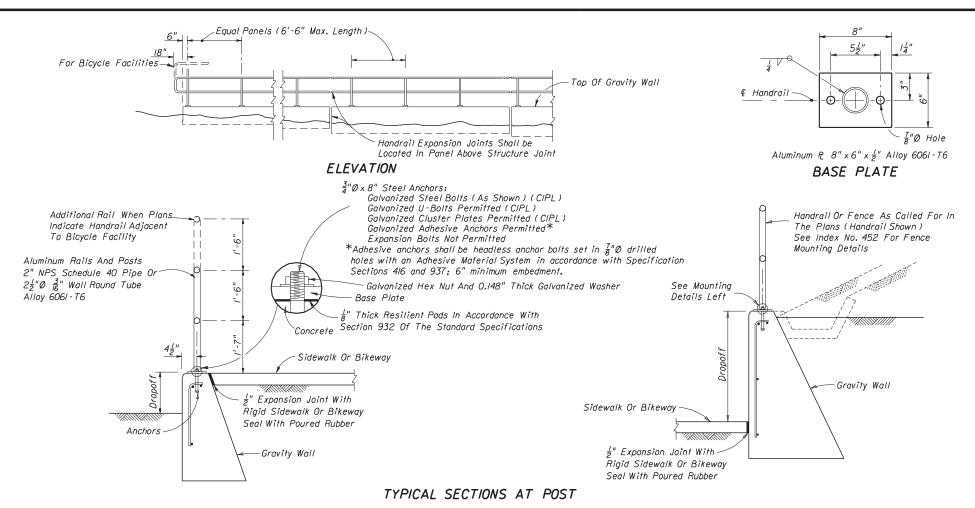
- Gravity walls constructed as extensions of reinforced concrete retaining walls, except walls of proprietary designs, shall have the same face texture and finish as the reinforced concrete retaining wall.
- Cost of reinforcing steel, face texture, finish and joint seal to be included in the contract unit price for Class I Concrete (Retaining Walls) CY.

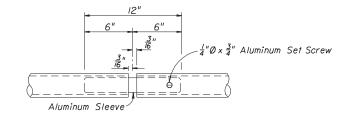
GRAVITY WALL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

ALUMINUM PIPE HANDRAILS, GRAVITY WALLS AND STEPS

	Names	Dates	Approve	d By	_D
Designed By			State Roadway Design Engineer		
Drawn By	CDR	02/68	Revision	Sheet No.	Index No.
Checked By	RHC	02/63	00	1 of 2	520





EXPANSION JOINT

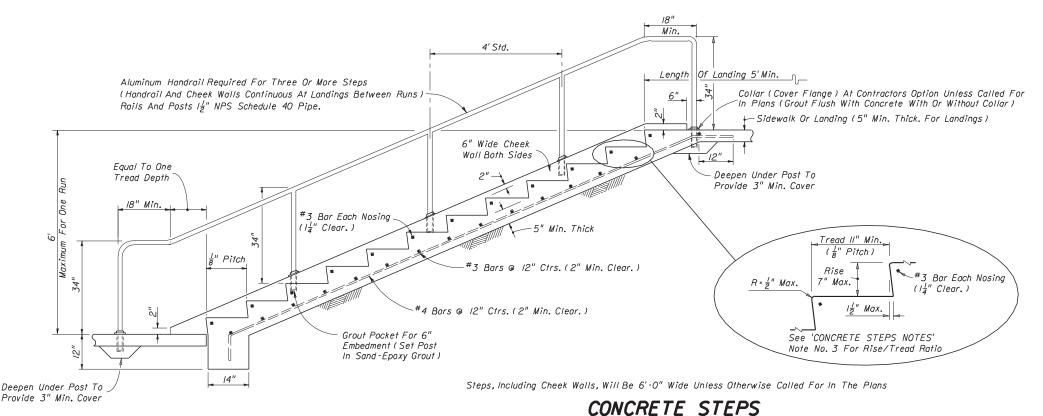
ALUMINUM PIPE HANDRAIL NOTES

- I. This handrail is applicable to mountings on walls and other roadway structures subject to pedestrian use where dropoffs do not exceed thirty inches (30"); and, applicable to select uses on sidewalk, within service areas and similar locations where foundation support and anchorage are adequate or can be provided.
- 2. All fixed joints to be either welded all around and ground smooth; or, commercially designed fixed joint systems (soldered, brazed, fused, bonded or shrink fitted) specified on the plans or approved by the Engineer. Mechanical joints other than expansion joints are not permitted unless specified on the plans or approved in writing by the Engineer. Posts shall be connected to base by weld only. Weld filler to be alloy ER5356, ER5556 or ER5183.
- 3. Anchor bolts shall be in accordance with ASTM A36 or A307. Nuts, washers, and bolts to be hot dip galvanized in conformance with ASTM A/53. After the nuts have been tightened, the anchor bolt thread at the top of the nut shall be punch distorted and coated with zinc compound.
- 4. Aluminum handrail shall be constructed in accordance with Section 5/5 of the Standard Specifications. Payment shall be full compensation for furnishing and installing handrail, including mounting hardware, and shall be paid for under the contract unit price for Pipe Handrail (Aluminum), LF.

DESIGN NOTES

I. For dropoffs that exceed thirty inches (30"), handrails for customary applications are provided in Structures Standard Index No. 850. For unusal site conditions where handrails are needed, the handrail is to be designed site specific by the responsible engineer.

ALUMINUM PIPE HANDRAIL ON GRAVITY WALLS FOR DROPOFFS >10" AND ≤2'-6"



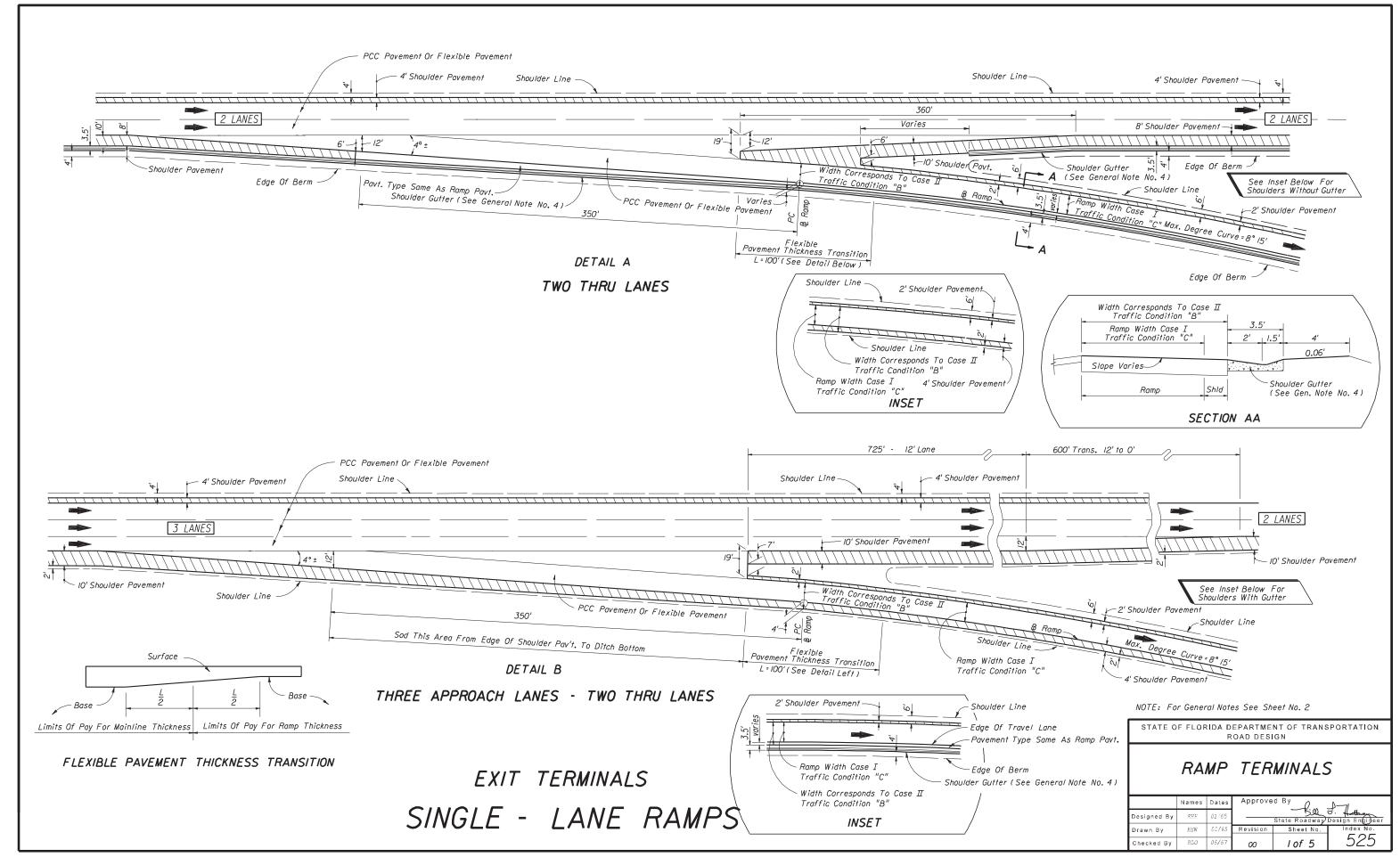
CONCRETE STEPS NOTES

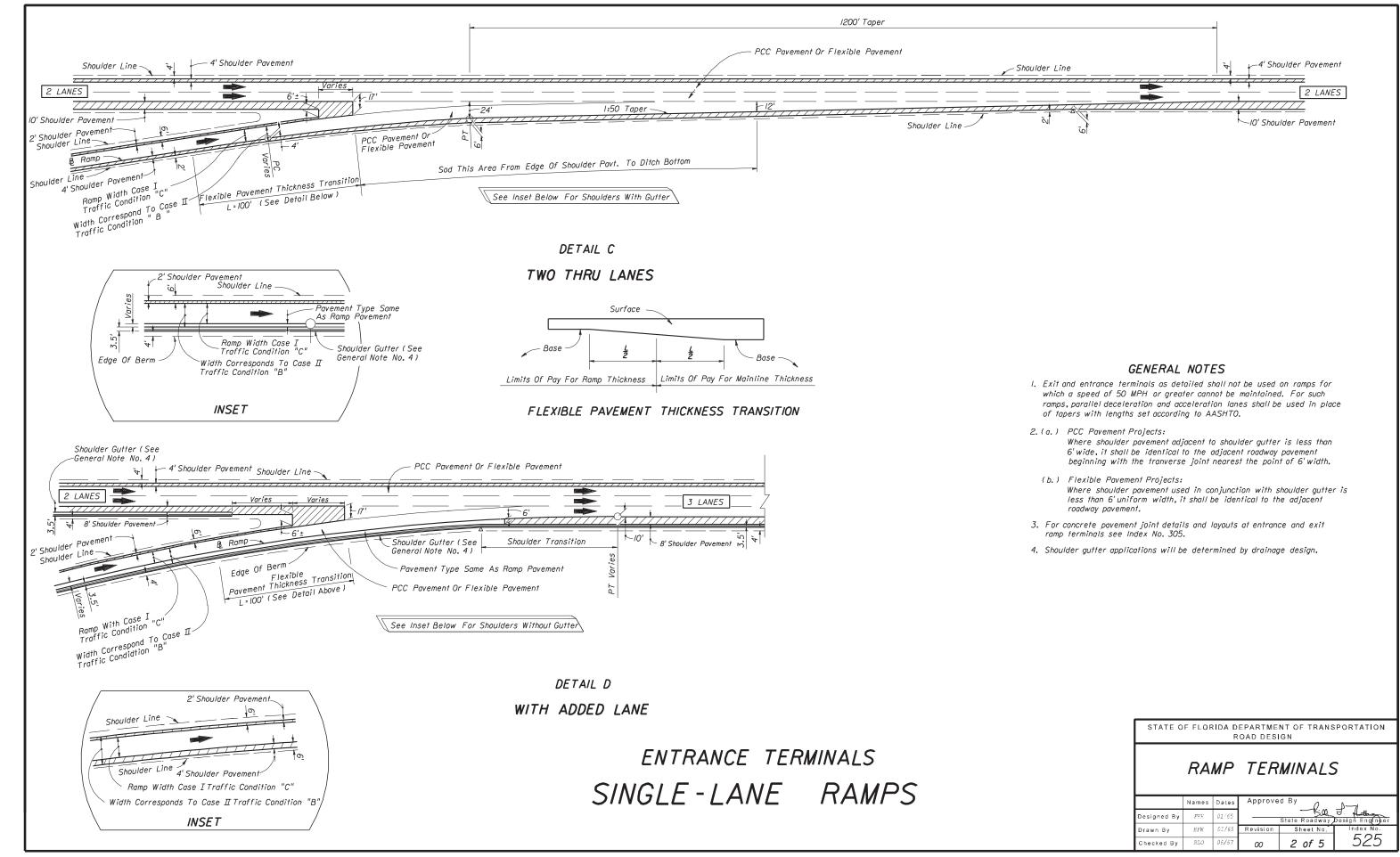
- Step and handrail design shown is for soil supported steps without adjacent dropoffs; do not use for suspended (structural) steps or stairway.
- 2. 12 risers maximum between landings.
- 3. Ratio of riser height to tread depth: 2R'' + T'' = 26''.
- 4. For steps parallel to and adjoining walls, delete adjoining cheek wall and mount handrail to wall at height and length shown.
- Aluminum handrail shall be constructed in accordance with Section 5/5 of the Standard Specifications. Payment shall be full compensation for furnishing and installing handrail, including mounting hardware, and shall be paid for under the contract unit price for Pipe Handrail (Aluminum), LF.
- Cost of concrete steps, landings and cheek walls shall be paid for under the contract unit price for Class I Concrete (Miscellaneous), CY. Cost of reinforcing steel shall be paid for under the contract unit price for Reinforcing Steel (Miscellaneous), LB.

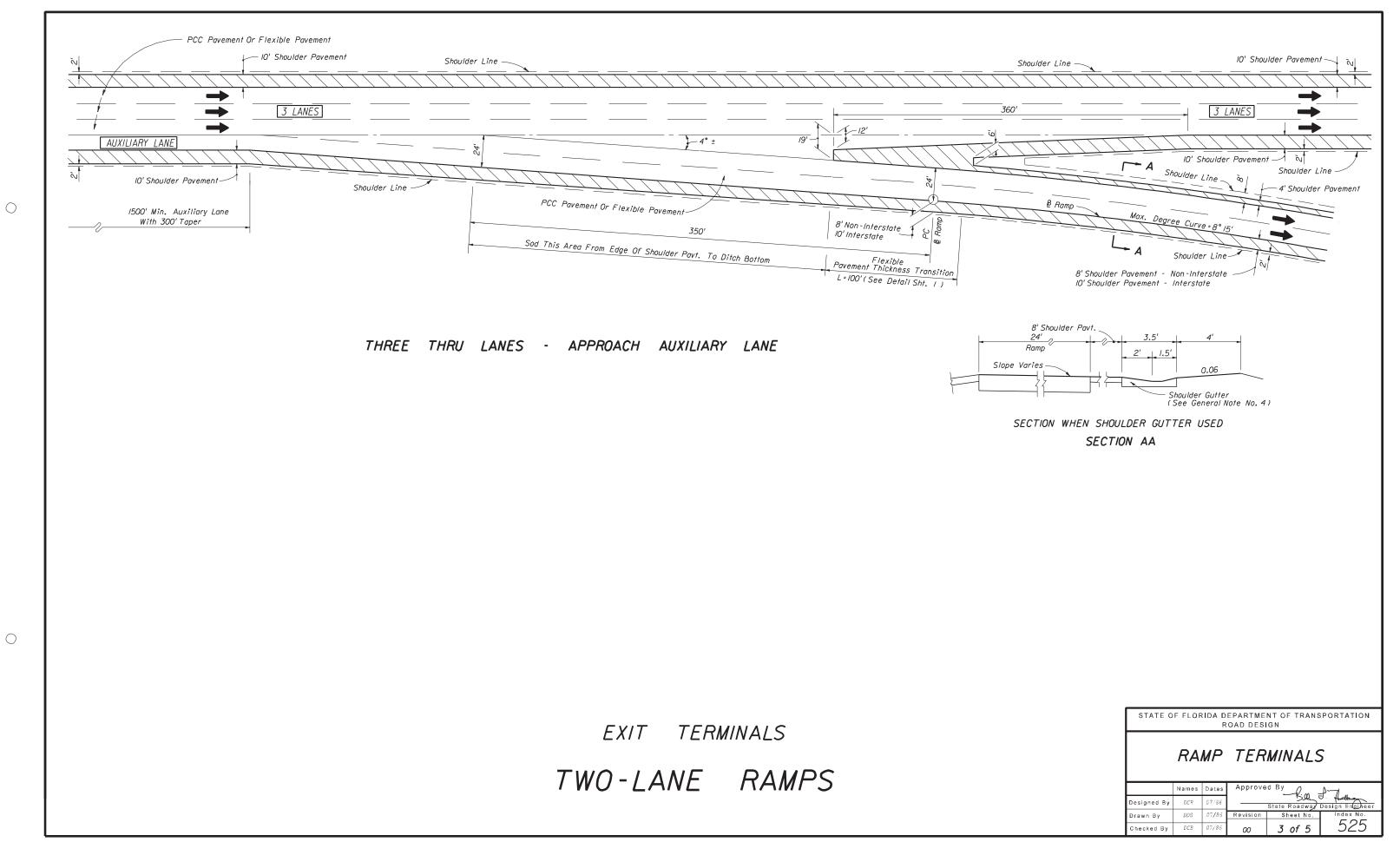
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

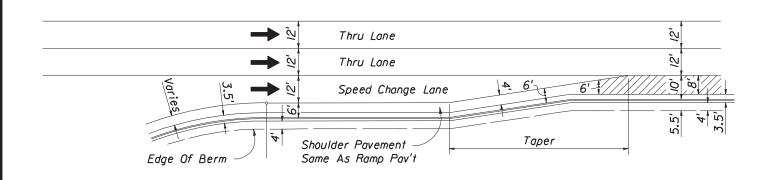
ALUMINUM PIPE HANDRAILS, GRAVITY WALLS AND STEPS

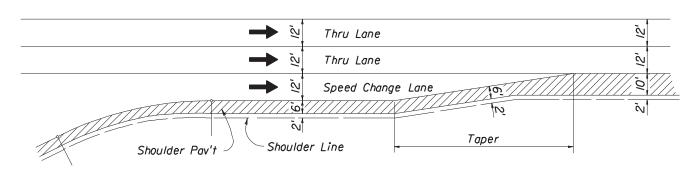
	Names	Dates	Approve	d By	D-71				
Designed By			State Roadway Design Engineer						
Drawn By	CDR	02/68	Revision	Sheet No.	Index No.				
Checked By	RHC	02/63	00	2 of 2	520				





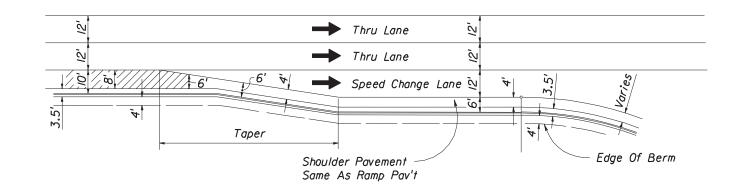


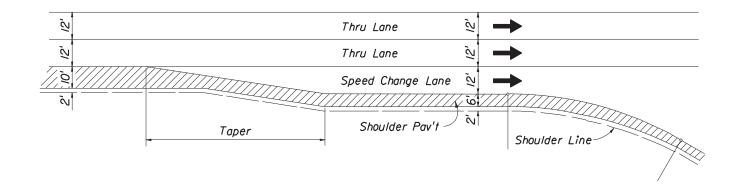




ACCELERATION LANE WITH SHOULDER GUTTER

ACCELERATION LANE WITHOUT SHOULDER GUTTER





DECELERATION LANE WITH SHOULDER GUTTER

DECELERATION LANE WITHOUT SHOULDER GUTTER

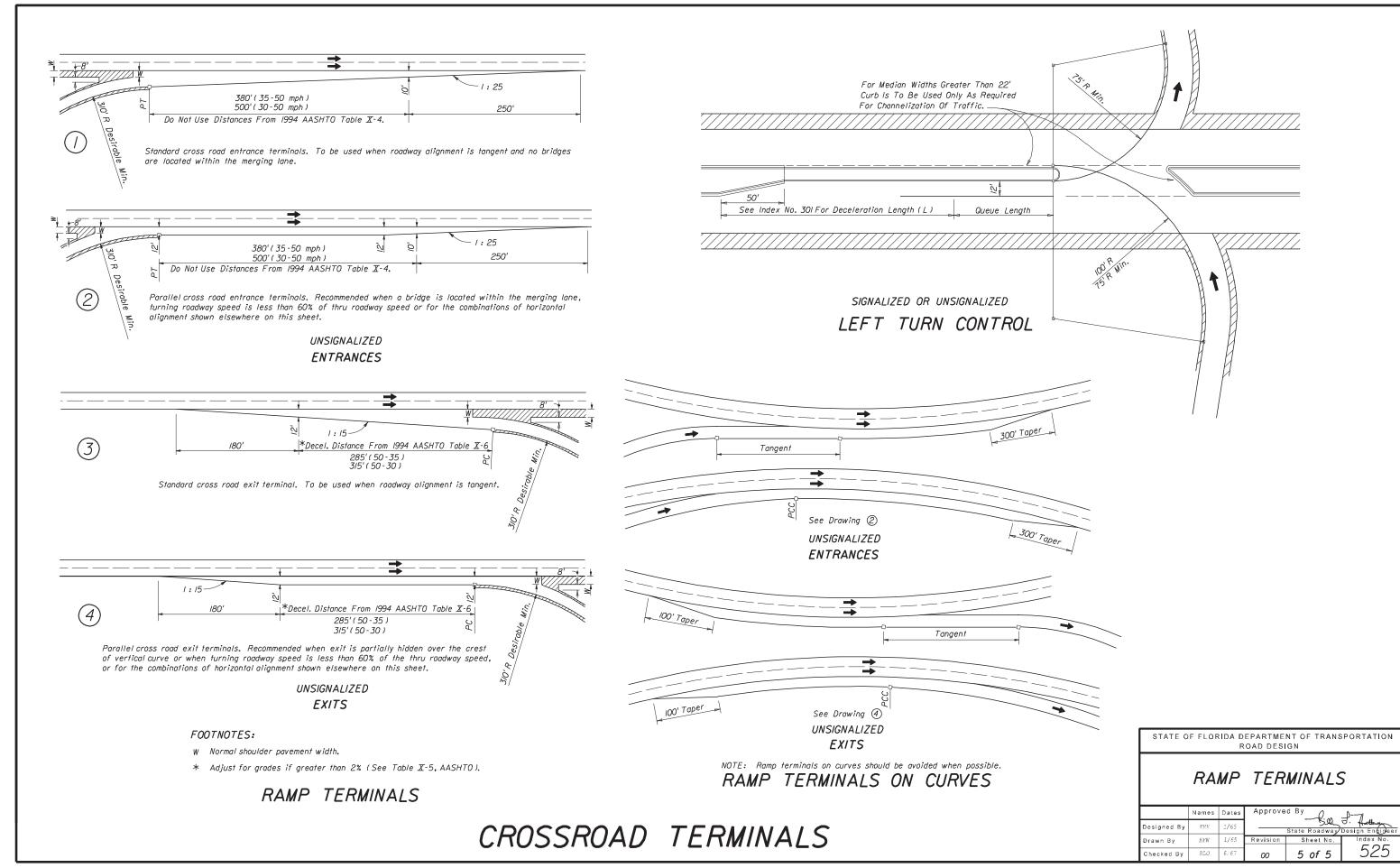
SHOULDER TREATMENT AT SPEED CHANGE LANES AT EXPRESSWAY RAMP TERMINALS

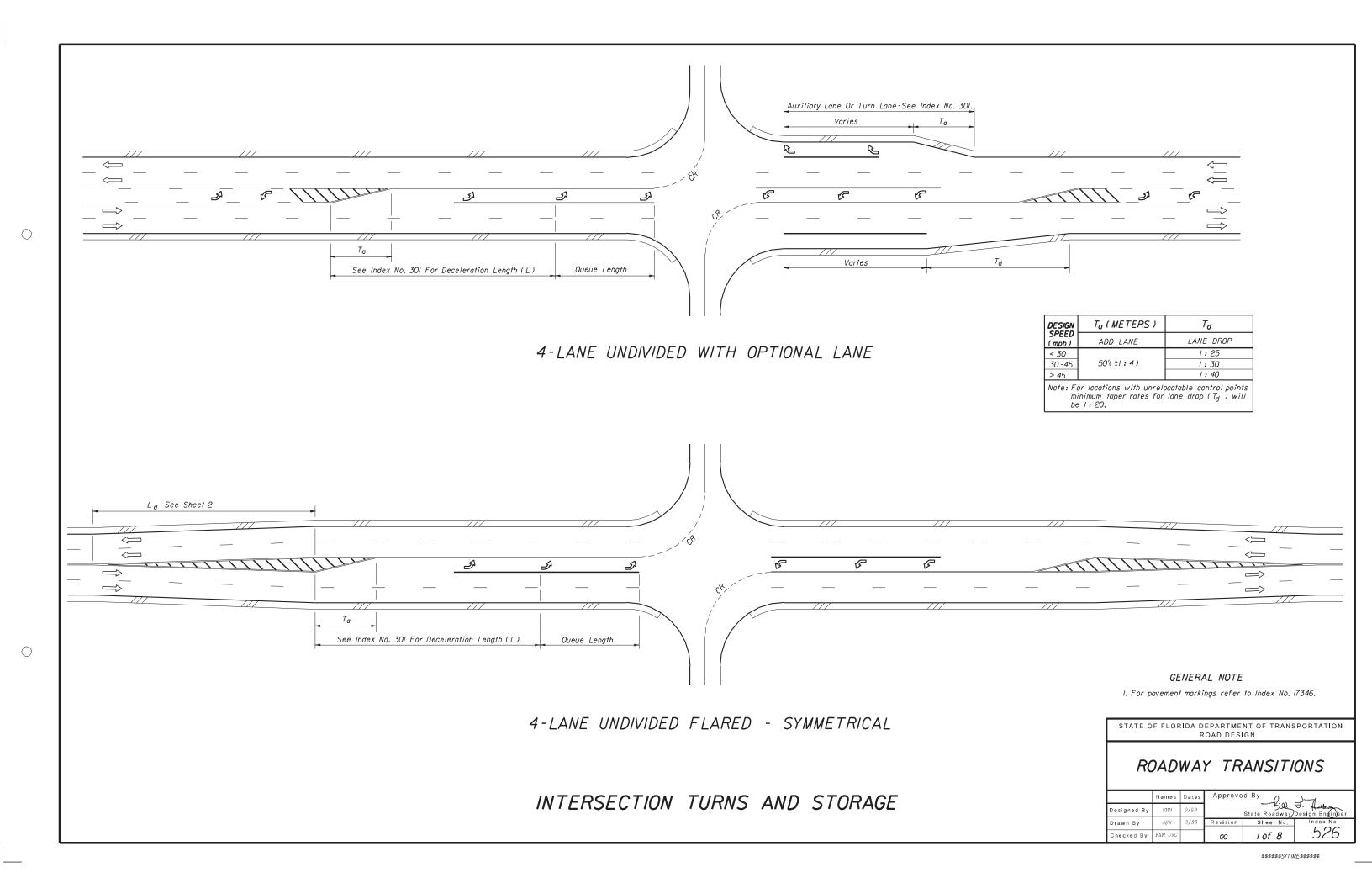
EXPRESSWAY RAMP TERMINALS

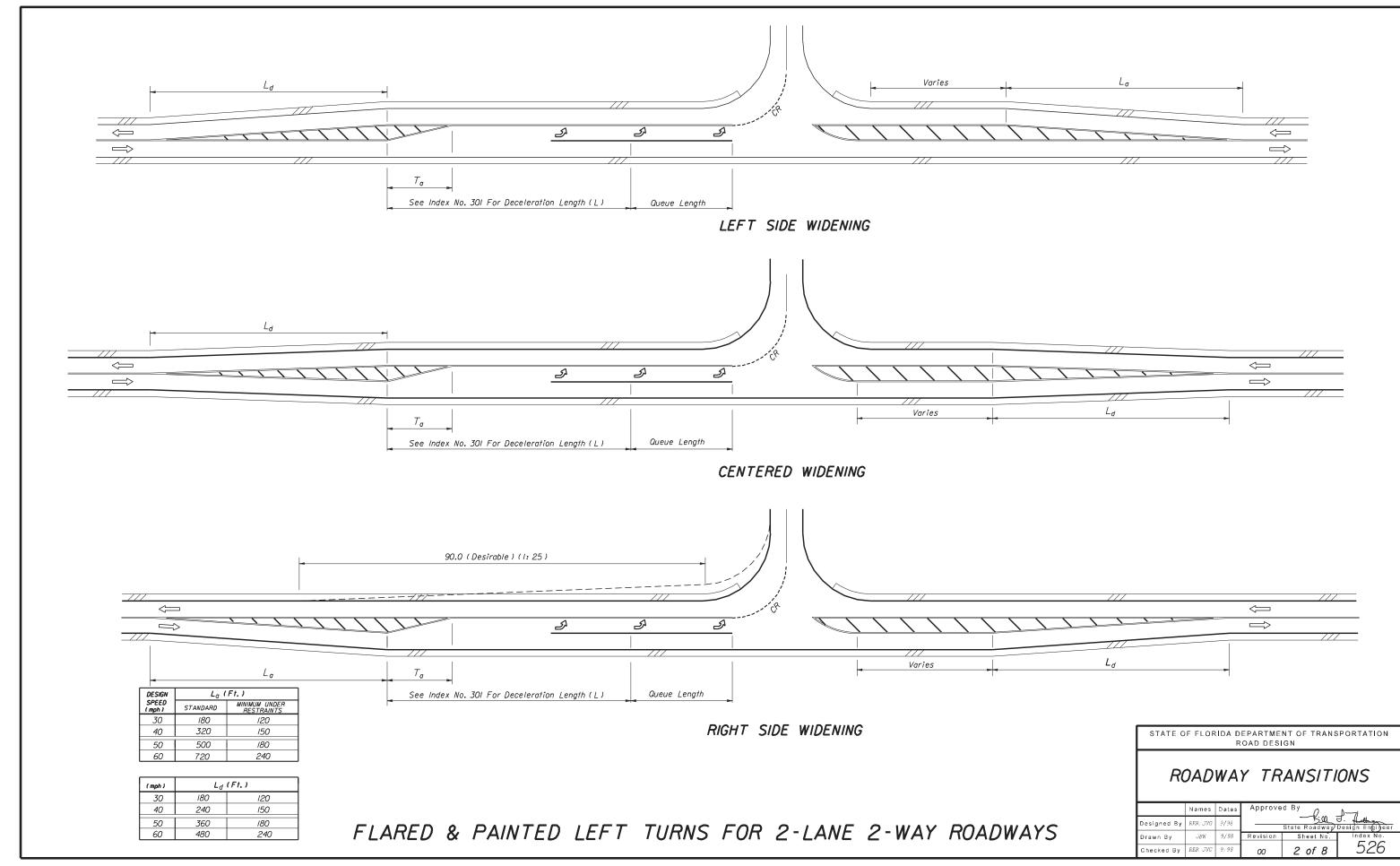
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

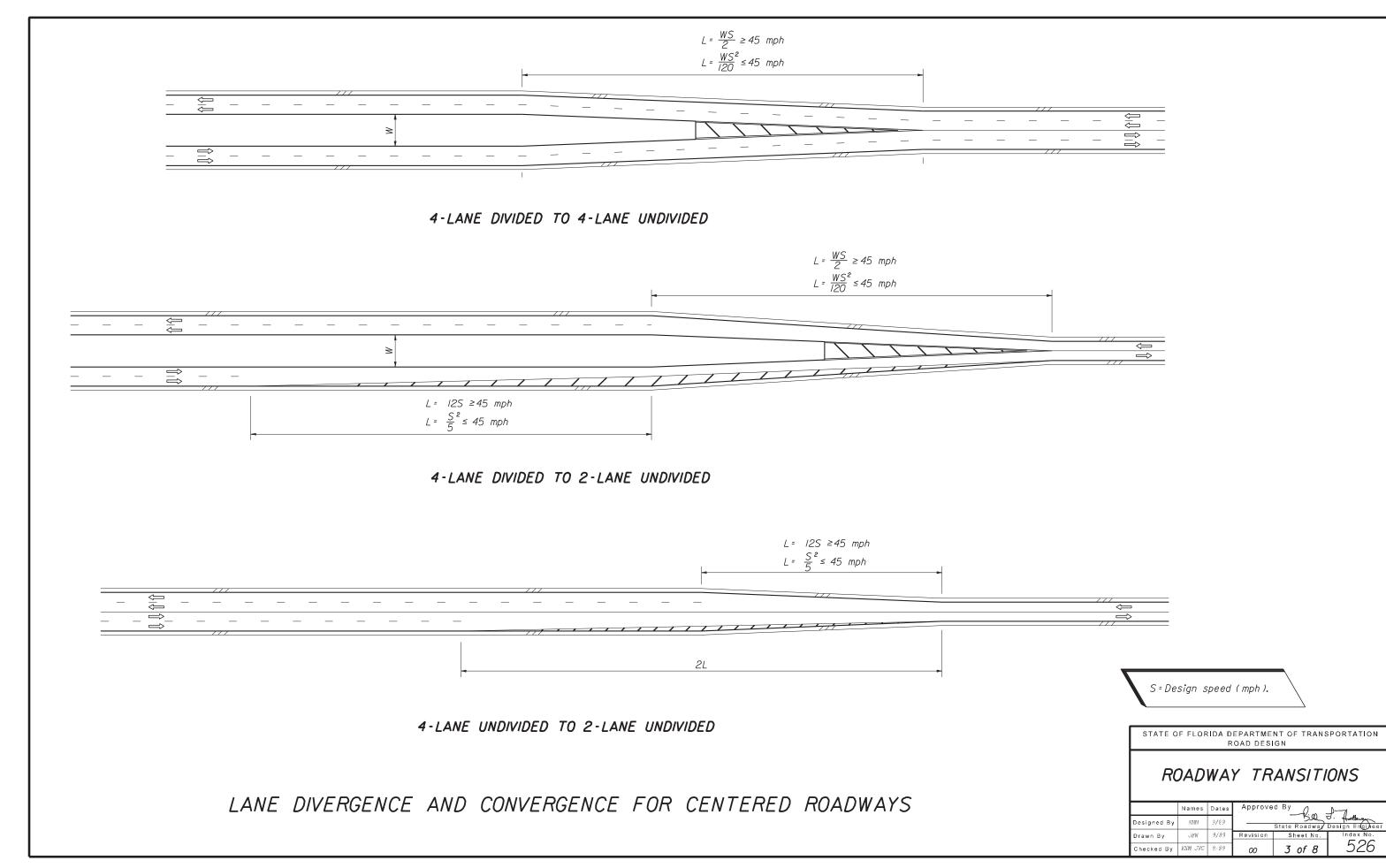
RAMP TERMINALS

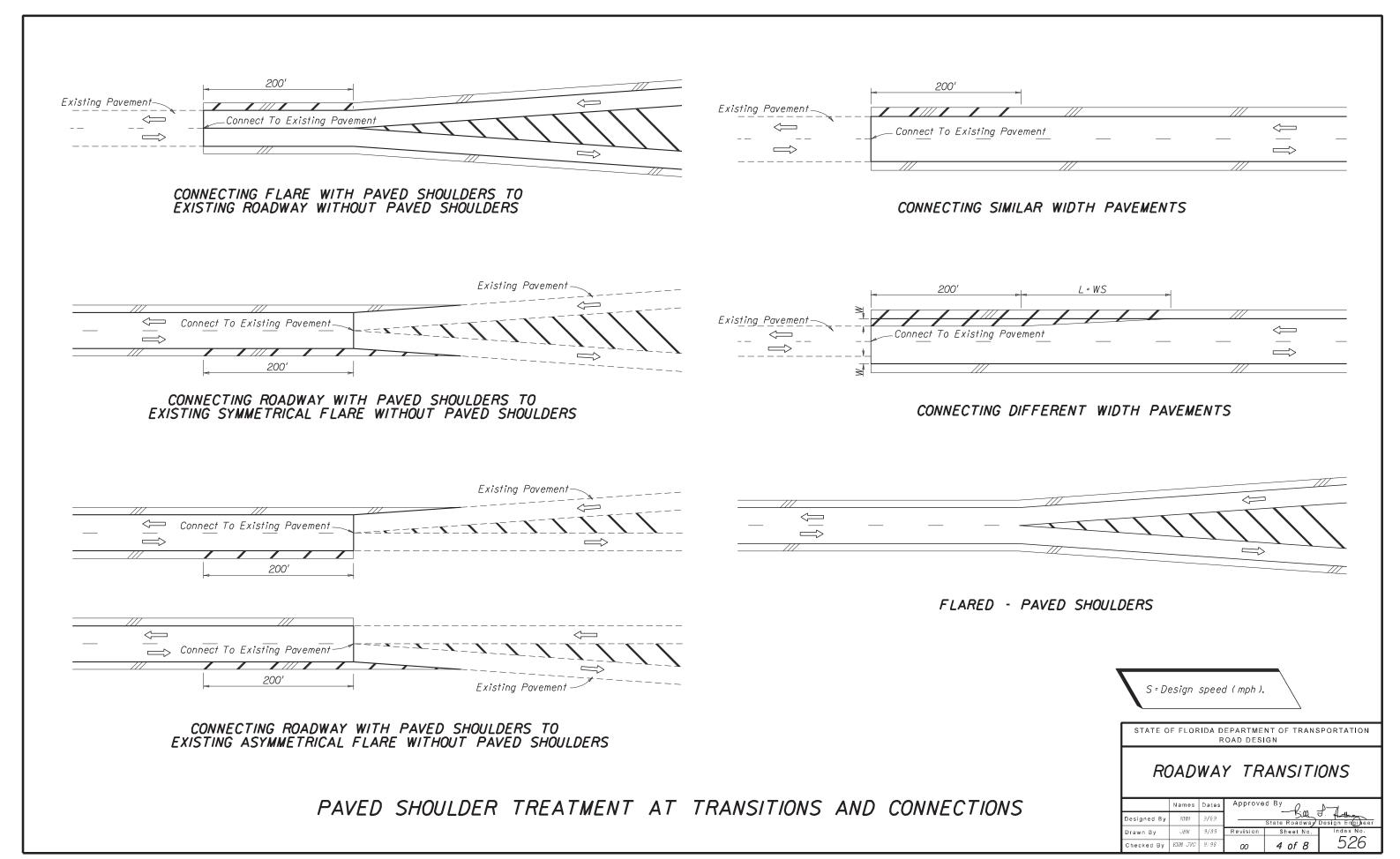
Designed By REE 01/65 State Roadway Design Entitleer Drawn By REW 01/65 Revision Sheet No. Index No. Checked By REO 06/67 00 4 of 5 525

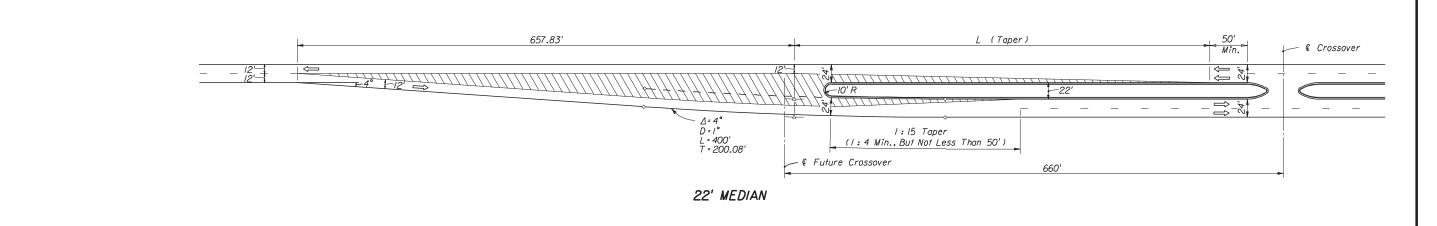


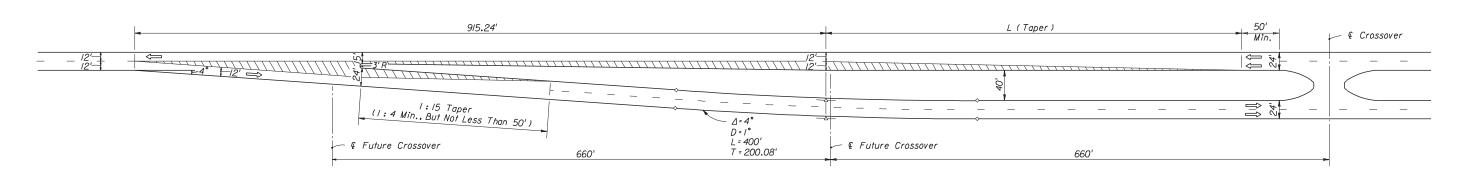




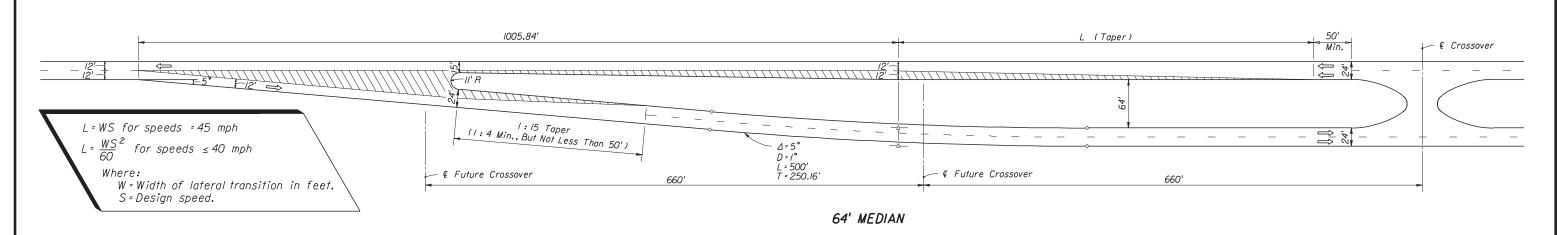








40' MEDIAN



NOTES FOR SHEETS 5 THRU 8

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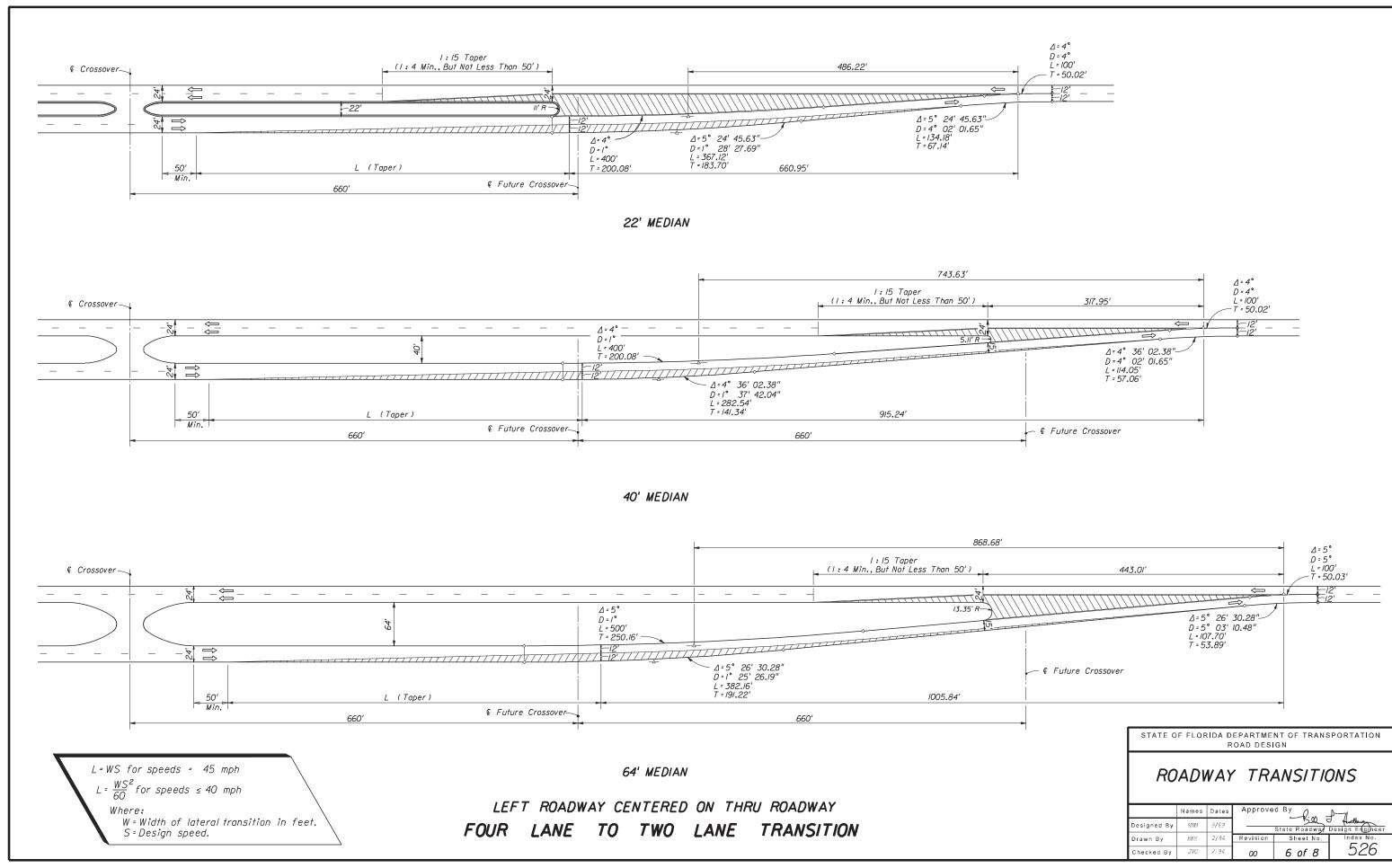
- I. The transition details as represented on sheets 5 thru 8 are intended as guidelines only. The transition lengths, curve data, nose radii and offsets are valid only for tangent alignment, design speeds ≤45 mph, the median widths and lane widths shown.
- 2. Approach lane departures (Δ =5°) are suitable for design speeds up to 60 mph. Interior curves (D=1°) are suitable for normal crown for design speeds up to 50 mph. Merging curves (D \geq 5°) will require superelevation.
- 3. The geometrics of these schemes are associated with the standard subsectional spacing for sideroads, but in any case will require modification to accommodate sideroad location, multilane and/or divided sideroads, oblique sideroads, crossover widths, storage and speed change lane requirements, and, other related features.

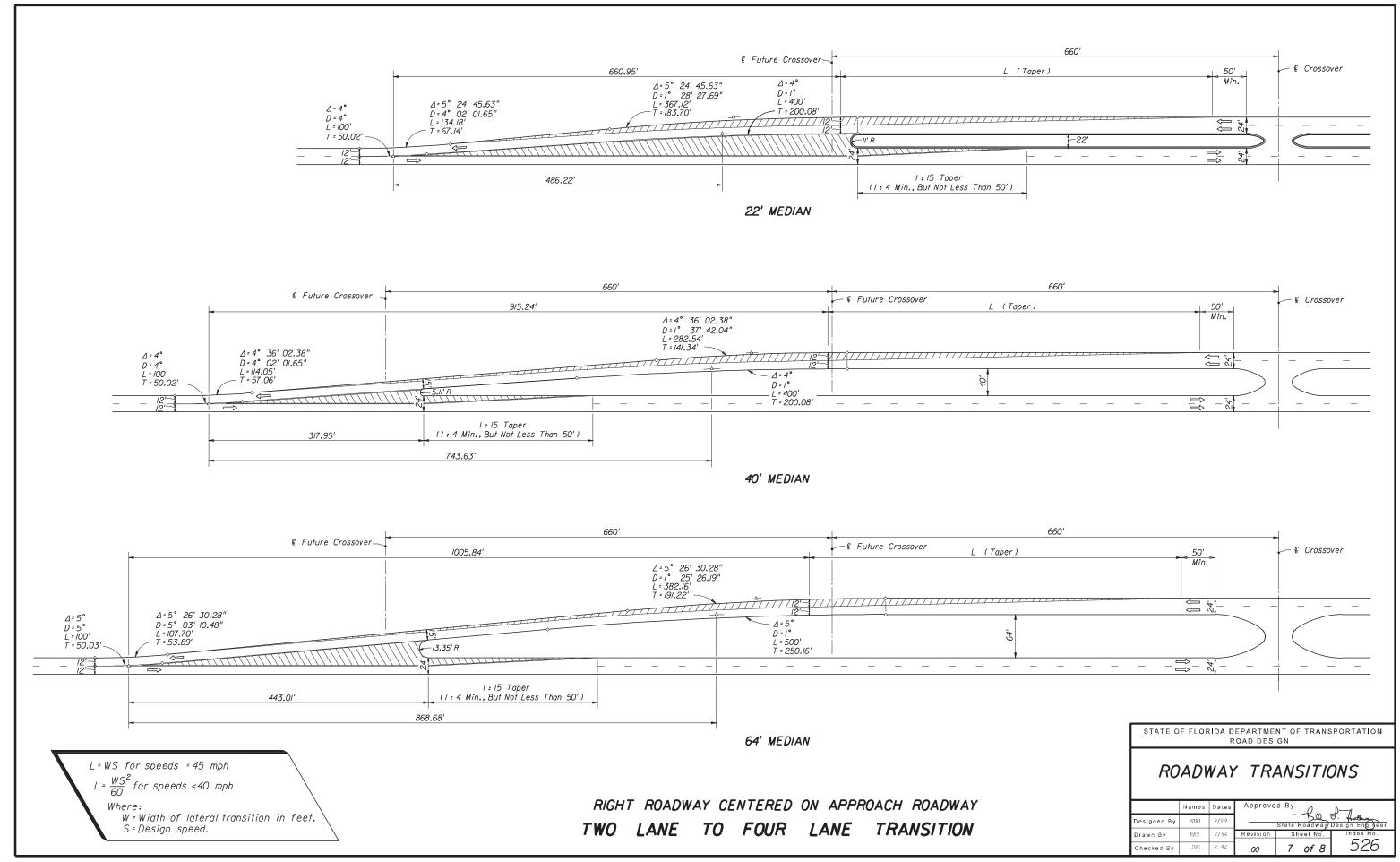
TWO LANE TO FOUR LANE TRANSITION

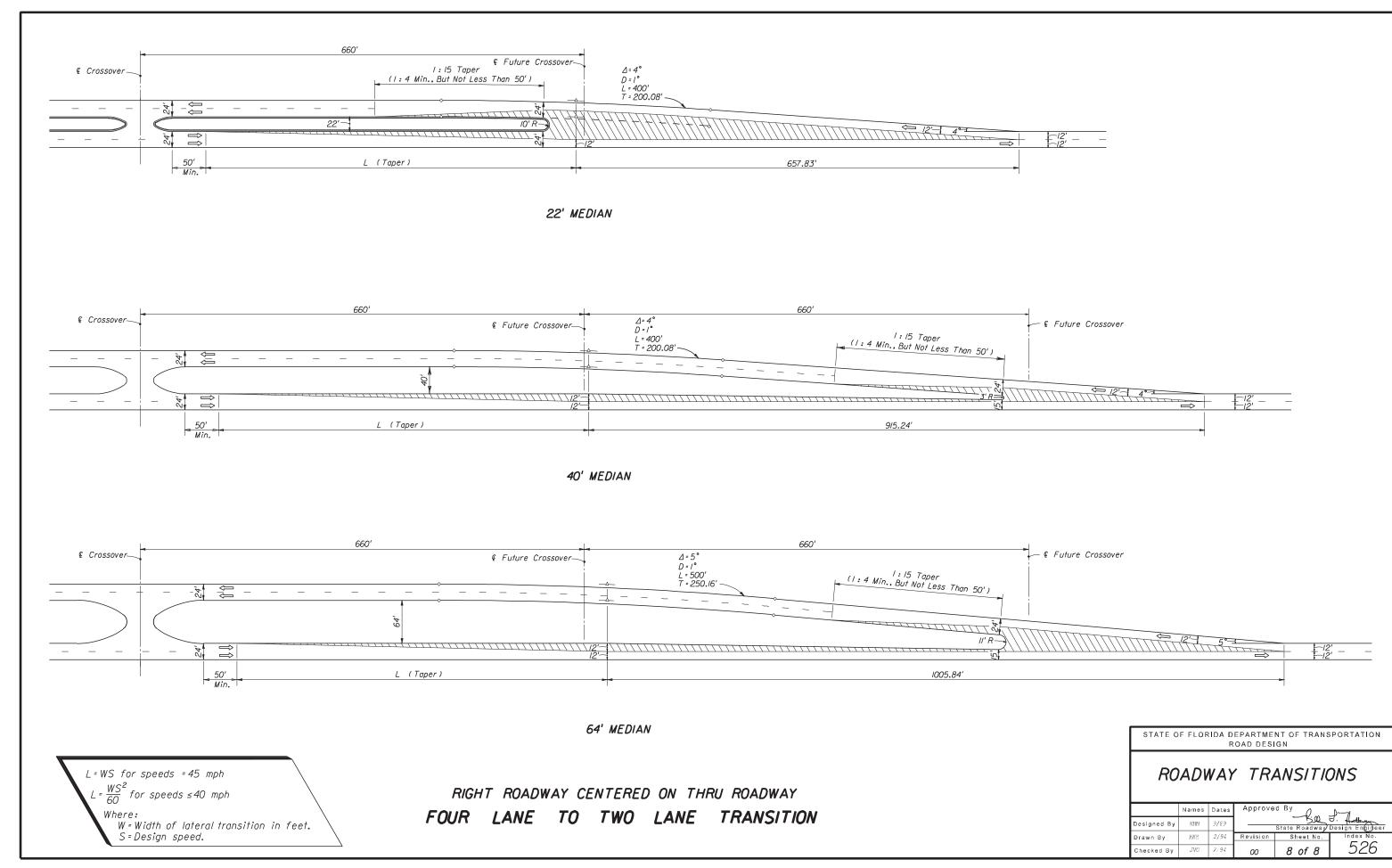
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

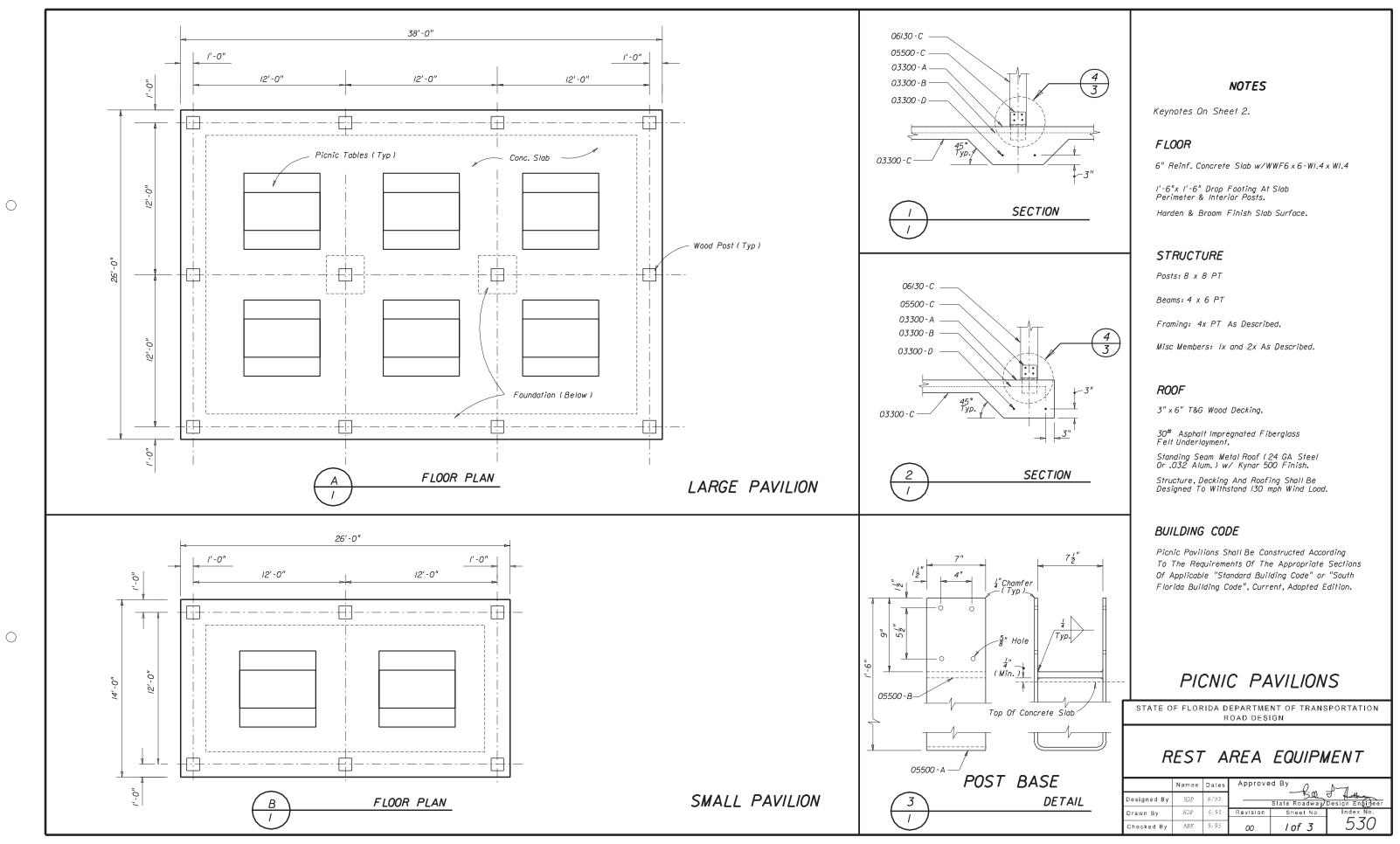
ROADWAY TRANSITIONS

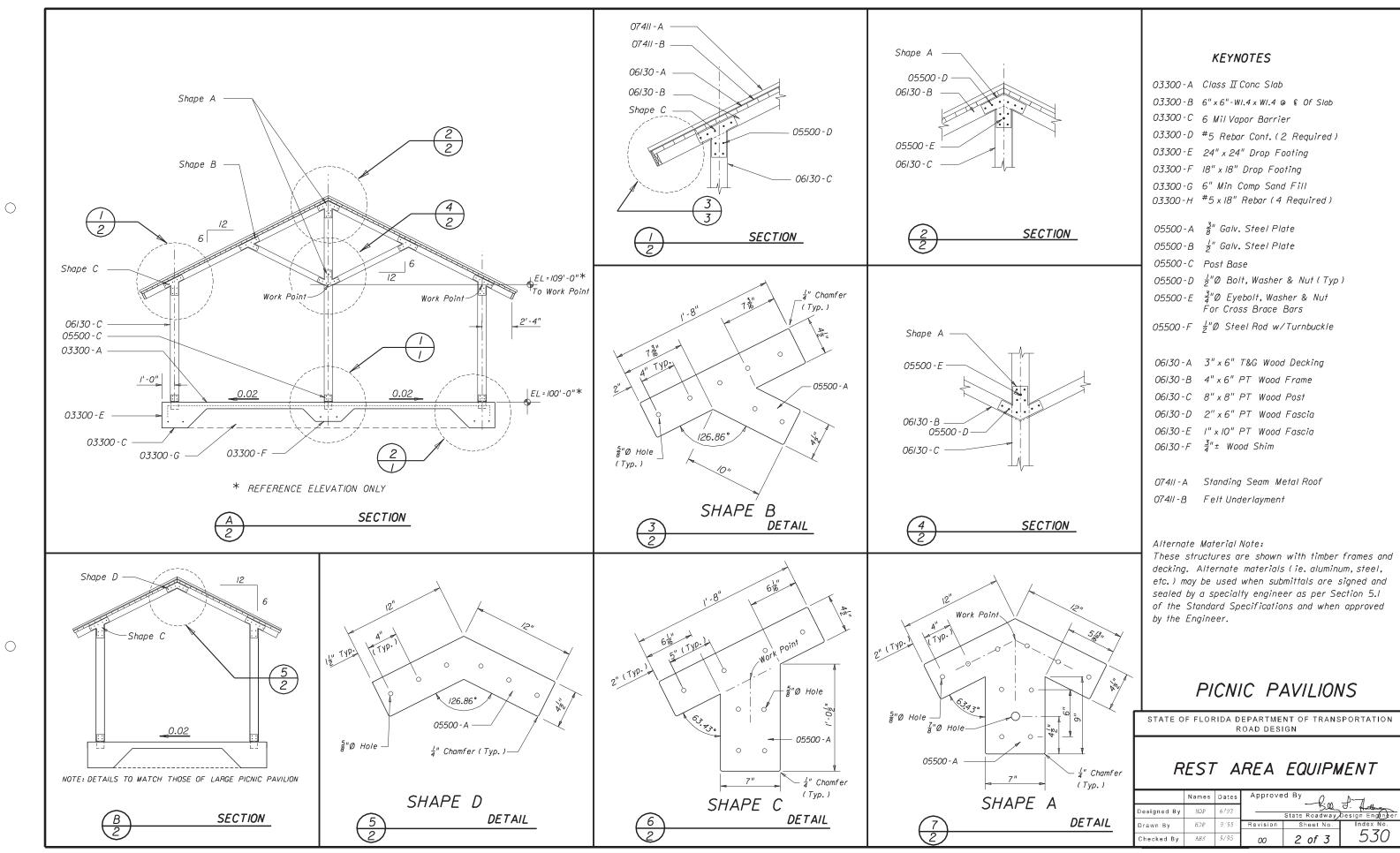
	Names	Dates	Approve	d By	D-71
Designed By	KNM	9/89		State Roadway	Design Engineer
Drawn By	HKH	2/94	Revision	Sheet No.	Index No.
Checked By	JVG	2/94	00	5 of 8	526

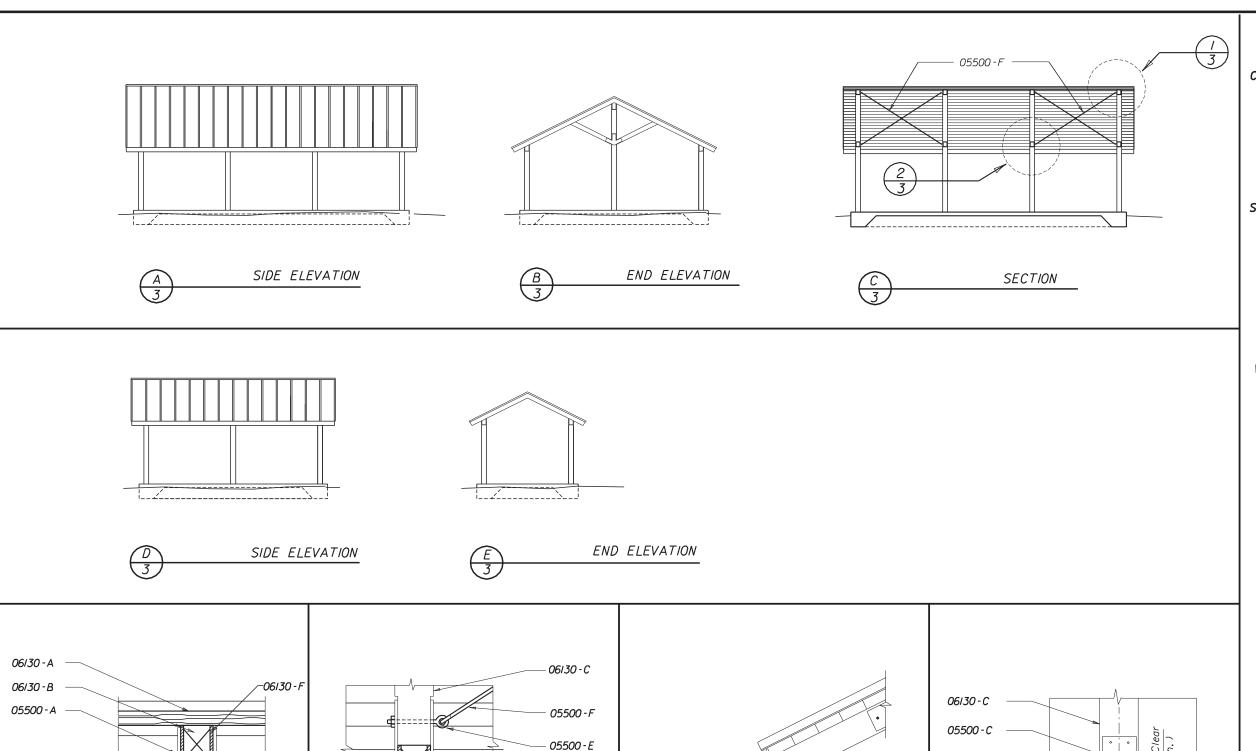












-05500 - A

-06130 -B

DETAIL

Notch 06/30-C To

Accomodate Steel Plates

06130 - F -

DETAIL

 \bigcirc

 \bigcirc

05500 - E

05500 - F

06130 - C

Notch 06/30 - C To Accomodate Steel Plates

SPECIFICATIONS

Keynotes On Sheet 2.

CONCRETE

Concrete: FDOT Class II.

Reinforcing Bars: ASTM A6/5/A6/5M, Grade 400.

Welded Wire Fabric: ASTM A-185.

Vapor Barrier: Black 6-Mil Polyethylene.

STEEL

Galvanized Steel Plate: Steel Plate ASTM A446 With G90 Zinc Coating.

Galvanized Fasteners: High-Strength Bolts And Nuts, ASTM A325 With G90 Zinc Coating.

Galvanize Shapes After Fabrication, Make Field Repairs To Galvanizing With High Zinc Dust Content Paint, Complying With SSPC-Paint-20.

WOOD

Comply With American Institute For Timber Construction AITC 108, "Standard For Heavy Timber Construction."

For Solid Wood Decking, Comply With AITC II2, Standard For Tongue And Groove Heavy Timber Standard."

Species: Douglas Fir, Hem-fir, Or Southern Pine, At Fabricator's Option.

Preservative Treatment: Pressure Treat Fabricated Members With Waterborne Solution For Above Ground Use, Complying With AWPA C2.

Wood Decking: Predrill Decking At 30" Centers For Lateral Spiking To Adjacent Units. Spikes To Be 20d Galvanized Common.

PICNIC TABLES

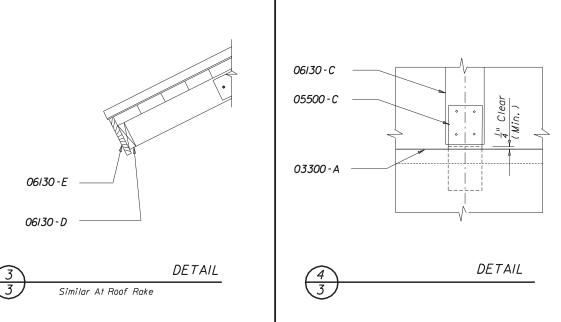
Picnic Tables And Benches Shall Be 6' x 6' w/Heavy Galvanized Pipe Frames And Recycled Plastic Wood Seats And Table Tops. All Tables Shall Be Of Walk Thru Design Suitable For Exterior Locations. Tables At Accessible Pavilions Shall Meet The Requirements Of The Americans With Disabilities Act (ADA) Accessibility Guidelines.

PICNIC PAVILIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

REST AREA EQUIPMENT

	Names	Dates	Approve	d By	A -71				
Designed By	HDP	6/95	State Roadway Design Engineer						
Drawn By	EDP	9/95	Revision	Sheet No.	Index No.				
Checked By	ABK	9/95	00	3 of 3	530				



GENERAL NOTES

- I. The location and construction of mailboxes shall conform to the rules and regulations of the United States Postal Service as modified by this design standard.
- 2. Mailboxes will not be permitted on Interstate highways, freeways, or other highways where prohibited by law or regulation.
- 3. The contractor shall give the Postmaster of the delivery route(s) written notice of project construction 7 days prior to the beginning of work, with Saturdays, Sundays and Holidays excluded.

The Contractor shall furnish and install one mailbox in accordance with this design standard at each mail patron delivery location and maintain the box throughout the contract period. The Contractor shall apply box numbers to each patron box in accordance with identification specifications of the Domestics Mail Manual of the U. S. Postal Service; where local street names and house numbers are authorized by the Postmaster as a postal address, the Contractor shall inscribe the house number on the box; if the box is located on a different street from the patrons residence, the Contractor shall inscribe the street name and house number on the box.

The Contractor shall coordinate removal of the patrons existing mailboxes. Immediately after installing the new mailboxes the Contractor must notify each "Mail Delivery Patron" by Certified Mail that removal of the existing mailboxes must be accomplished in 2l days after receipt of notices. Patrons shall have the option of removing their existing mailboxes or leaving the mailboxes in place for removal by the Contractor; removal by the Contractor shall be included in the contract unit price for Mailbox, Each. The Contractor shall dispose of mailboxes and supports in areas provided by him.

Reuse of existing mailboxes by the Contractor will not be a requirement under any construction project; however where an existing mailbox meets the design requirements of this standard and is structurally and functionally sound, the Contractor at his option may elect to reuse the existing mailbox in lieu of constructing a new mailbox. Any use of existing mailboxes must be approved by the Engineer.

4. Mailboxes shall be metal construction only, in traditional style only, and only in Size I as prescribed by the Domestic Mail Manual of the U. S. Postal Service (DMM).

Mailbox production standards, lists of approved manufacturers and suppliers of mailboxes, design approval and guidance may be obtained by writing to the Rural Delivery Division, Delivery Service Department, Operations Group, USPS Headquarters, Washington, DC 20260.

5. Mailboxes shall be located on the right-hand side of the roadway in the direction of the delivery route, except on one-way roads and streets where they may be placed on the left-hand side.

Mailboxes on rural highways shall be set with the roadside face of the box offset from the edge of the traveled way a minimum distance of the greater of the following:

- (a) Shoulder width plus 8" to 12".
- (b) 10' for ADT over 10,000 vpd.
 - 8' for ADT 100 to 10,000 vpd.
 - 6' for ADT under 100 vpd
 - 2'-6" for low speed and ADT under 100 vpd.

When a mailbox is installed within the limits of guardrail it should be placed behind the guardrail whenever practical.

Mailboxes on curbed highways, roads and streets shall be set with the face of the box between 6" and 12" back of the face of curb. If the sidewalk abuts the curb or if an unusual condition exists which makes it difficult or impractical to install or serve boxes at the curb, the Contractor with concurrence of the local postal authority may be permitted to install all mailboxes at the back edge of the sidewalk, where they can be served by the carrier from the sidewalk.

- 6. Mailboxes shall be set with the bottom of the box between 42" and 48" above the mail stop surface, unless the U.S. Postal Service establishes other height restrictions.
- 7. No more than two mailboxes may be mounted on a support structure unless the support structure and mailbox arrangements have been shown to be safe by crash testing and approved by the State Design Engineer, Roadways.

Neighborhood Delivery and Collection Box Units (NDCBU) are a specialized multiple mailbox installation that must be located outside the highway and street clear zones. The location of NDCBUs is the sole responsibility of the Postmaster for the delivery route under consideration.

8. Lightweight newspaper receptacles may be mounted below the mailbox on the side of the support post in conformance with the USPS Domestic Mail Manual. The mail patron shall be responsible for newspaper receptacle installation and maintenance.

9. Wood and steel support posts for both single and double mailbox mountings shall be embedded no more than 24" into the ground.

Concrete, block, brick, stone or other rigid foundation structure or encasement, either above or below the shoulder groundline, will not be permitted for mailboxes on rural highways. On urban roads and streets where mailbox support posts are set within rigid povement back of curb, the support posts shall be separated from the pavement by a minimum of I" of expansion material.

Support posts shall not be fitted nor installed with surface mount base plates.

10. At driveway entrances mailboxes shall be placed on the far side of the driveway in the direction of the delivery route.

At intersecting roads mailboxes shall be located 100' or more from the centerline of the intersecting road on the far side in the direction of the delivery route, with the distance increased to 200' when the route volume exceeds 400 vehicles per day.

II. Wood support posts shall be in conformance with the material and dimensional requirements of Section 952 and the treatment requirements of Section 955 of the Standard Specifications.

Steel support posts shall have an external finish equal to or better than two coats of weather resistant, air dried or baked, paint or enamel. Surfaces(s) shall be cleaned of all loose scale prior to finishing. The Postal Service prefers that posts be painted white, but other colors may be used when approved by the Engineer. When galvanized posts are used painting is not required.

Mounting brackets, plates, platforms, shelfs and accessory hardware surface finishes are to be suited to support post finish.

12. Mailboxes shall be paid for under the contract unit price for Mailboxes, Each. Payment shall be full compensation for boxes, posts and accessory items essential for installation in accordance with this standard; erection; adjustments to suit construction needs; and, for identification letters and numbers.

Payment shall be limited to one mailbox per patron address whether the mailbox is new, reused, salvaged, reset or relocated. Payment shall be per mailbox regardless of the number of mailboxes per support or grouping arrangement.

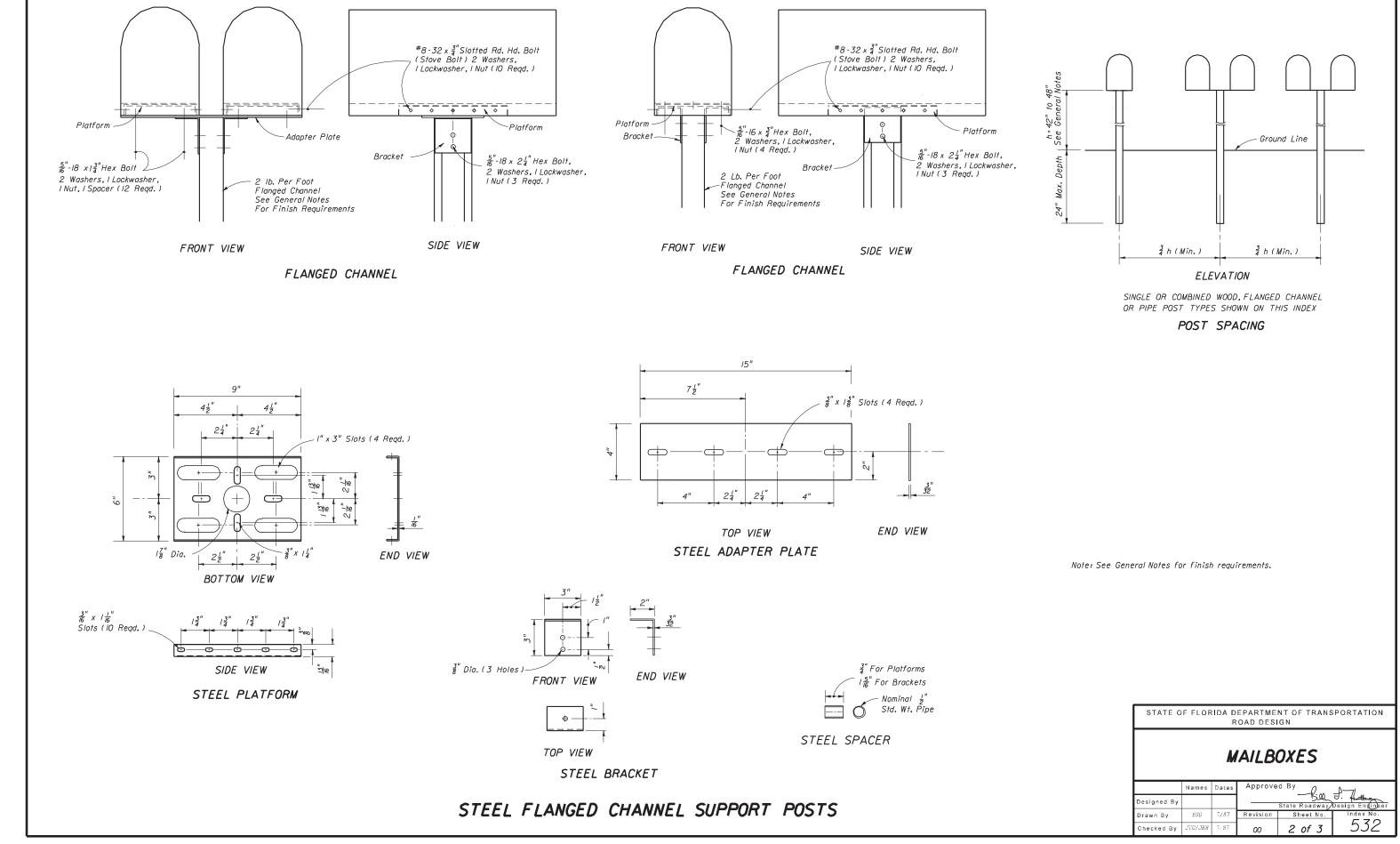
The above compensation shall include any work and cost incurred by the contractor for removal and disposal of existing mailboxes.

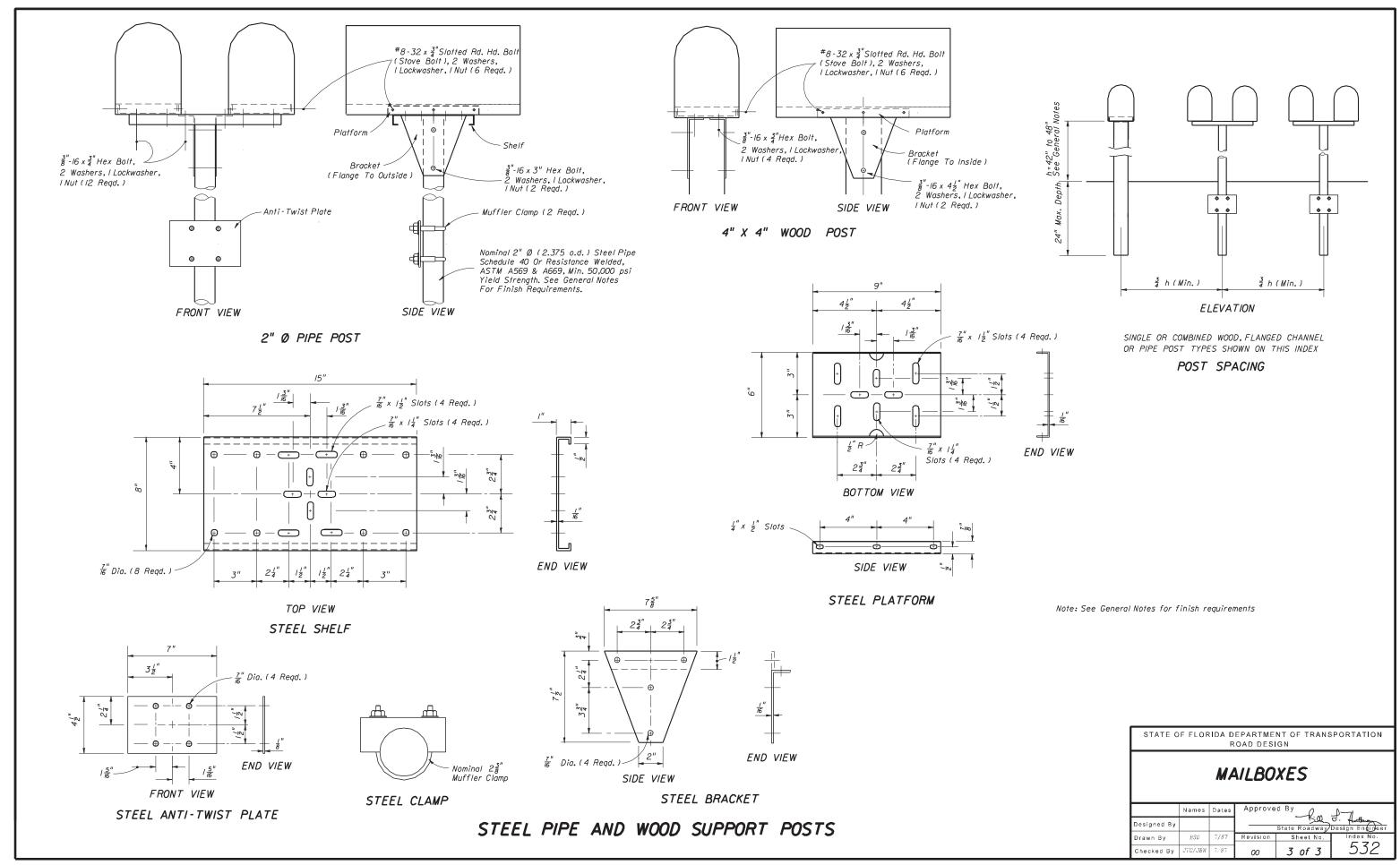
There shall be no payment participation for NDCBU furnishing, assembly, installation, resetting or relocation.

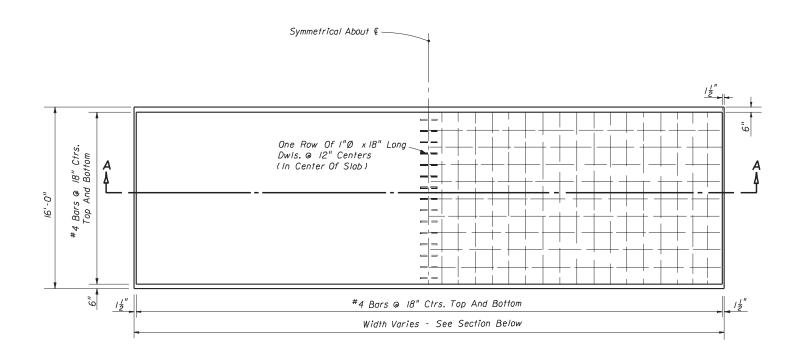
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

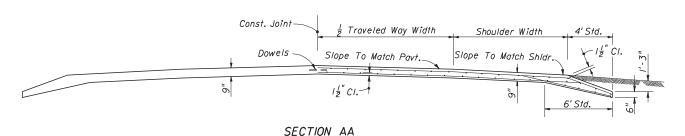
MAILBOXES

	Names	Dates	Approve	d By	1-7
Designed By				State Roadway	Design Engineer
Drawn By	HSD		Revision	Sheet No.	Index No.
Checked By	JVG/J3W		00	I of 3	532









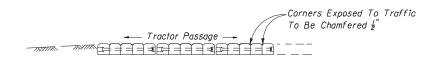
PLAN

Note: Class I concrete is to be used unless otherwise noted in plans or special provisions.

REINFORCED CONCRETE TYPE A

Width Vories-See Note Below

PLAN



SECTION BB

Note: Tractor crossing to be constructed to match pavement cross slope.

The number of mats required will vary with the pavement width.

A sufficient number of mats will be used so that the tractor crossing will extend a minimum of four feet (4') beyond roadway shoulders.

TREATED TIMBER

TYPE B

GENERAL NOTES

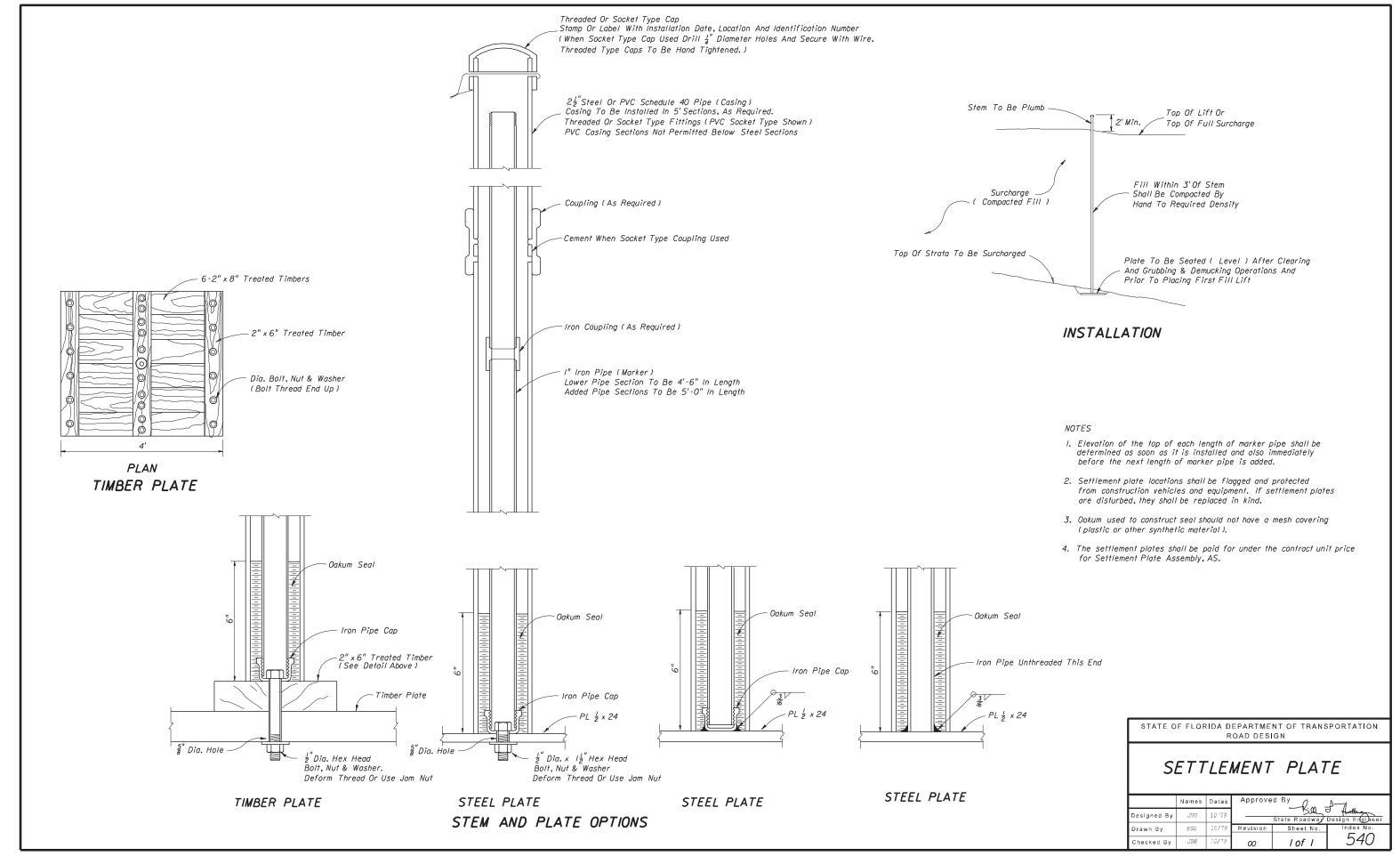
I. Tractor crossing shall be paid for under the contract unit price for Tractor Crossing, EA.

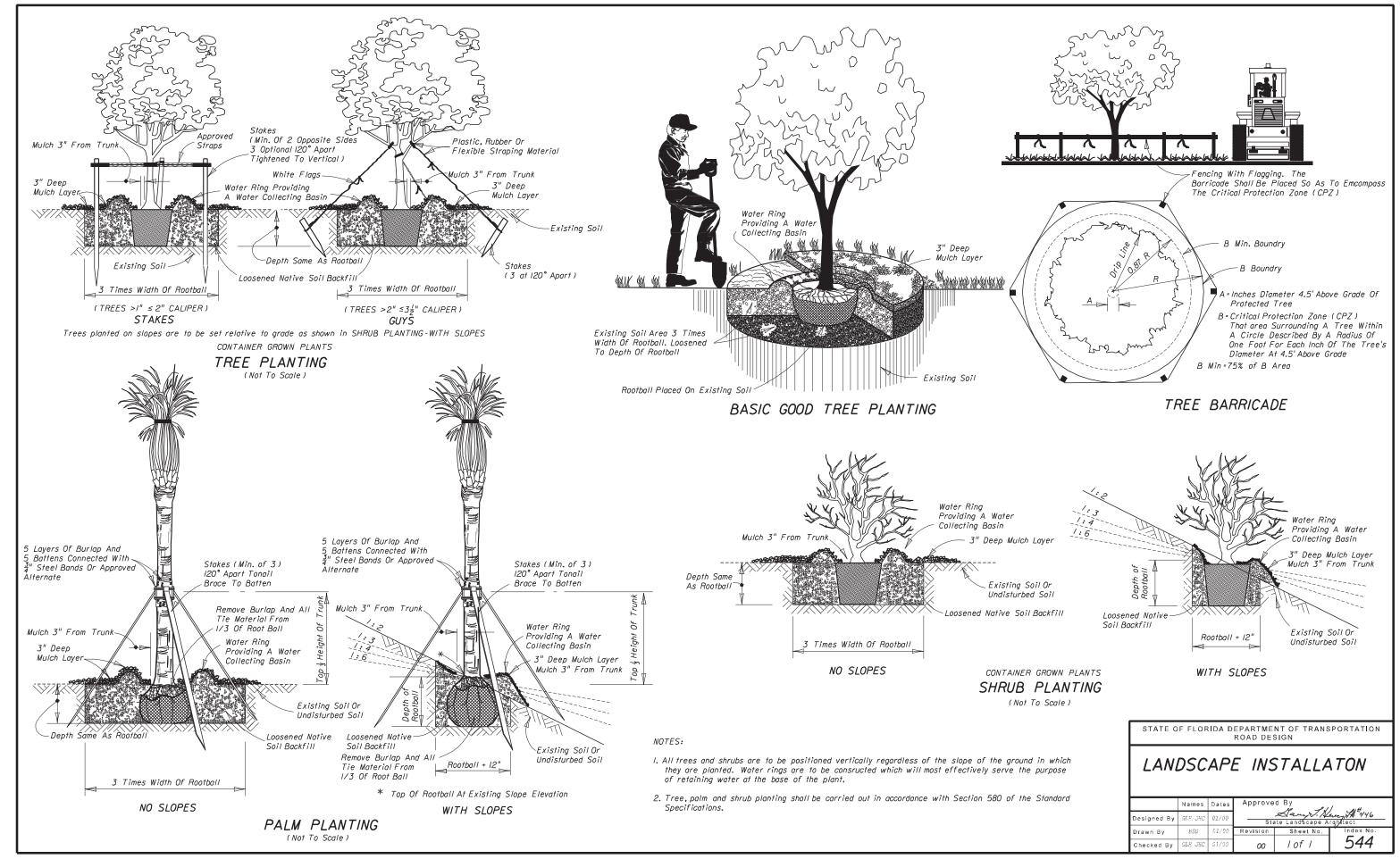
TRACTOR CROSSINGS

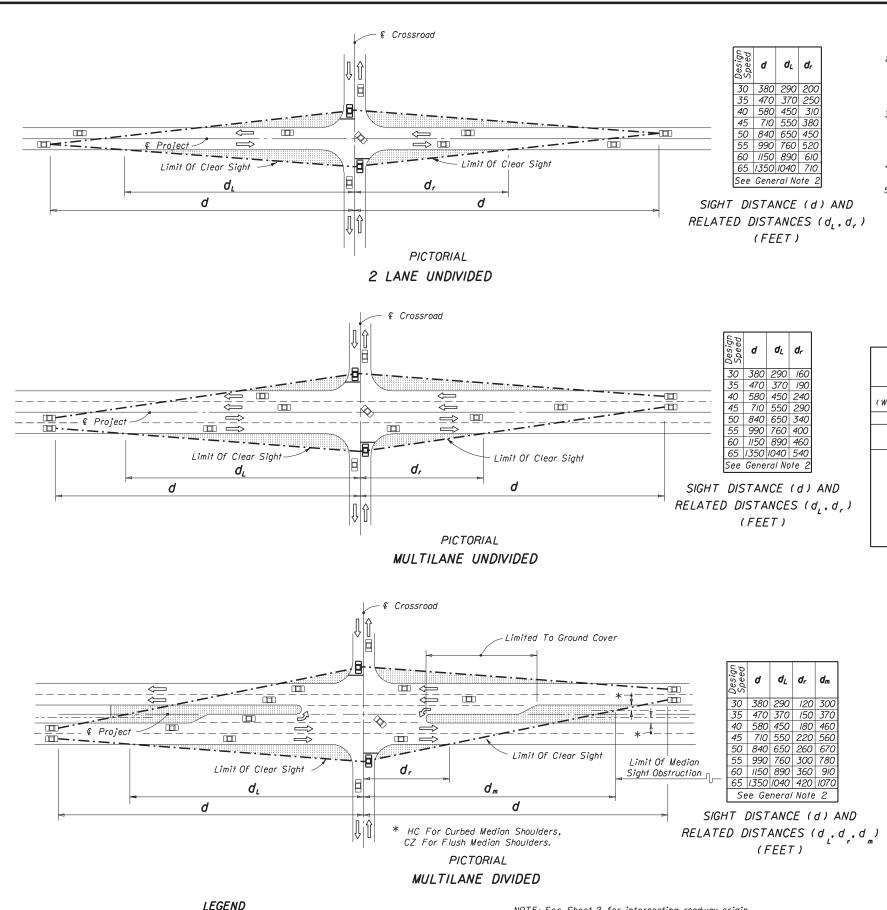
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRACTOR CROSSINGS

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Designed By				State Roadway	Design Engineer
Drawn By	LH	01/61	Revision	Sheet No.	Index No.
Checked By	CDD	01/61	00	I of I	535







Areas Free Of Sight Obstructions

NOTE: See Sheet 2 for intersecting roadway origin of clear sight and quadrant corner clips.

GENERAL NOTES

- I. Details apply to both rural and urban intersections under stop sign control or flashing beacon control. For full signal controlled intersections see Design Note No. 4 below.
- 2. Sight distance (d) applies to normal and skewed intersections (intersecting angles between 60° and 120°), and where vertical and/or horizontal curves are present. Sight distance (d) is measured along the major roadway from the center of the intersecting roadway. Distances d_L and d_r are measured from the centerline of the intersecting roadway to a point on the edge of the near side outer traffic lane on the major roadway. Distance d_m is measured from the centerline of the intersecting roadway to a point on the median clear zone limit or horizontal clearance limit for the far side roadway of the major roadway.
- 3. a. The limits of clear sight define a corridor throughout which a clear sight window must be preserved. See WINDOW DETAIL, Sheet 2.
- b. Clear sight must be provided between vehicles at intersection stop locations, and vehicles on the major roadway within dimension 'd'.
- c. Since observations are made in both directions along the line of sight, the reference datum between roadways is 3'-6" above respective payements.
- 4. Barrier systems within intersection sight corridors, where penetration into the sight window might occur, shall be located to provide the least adverse affect practical.
- 5. The corridor defined by the limits of clear sight is a restricted planting area. Drivers of vehicles on the intersecting roadway and vehicles on the major roadway must be able to see each other clearly throughout the limits of 'd'. If, in the Engineers judgement landscaping interferes with the line of sight corridor prescribed by these standards the Engineer may rearrange, relocate or eliminate plantings. Plants within the restricted areas are limited to selections as follows:
 - Ground Cover & Trunked Plants (Separate or Combined):
 - Ground Covers- Plant selection of low growing vegetation which at maturity does not attain a height greater than 18" below the sight line datum.
 - For ground cover in combination with trees and palms; the following heights below the sight line datum will apply: 24" for trees and palms $\leq ||||$ " dia.; and, ||8||" for sabal palms > ||||" $\leq ||8||$ " dia. (dia. -within Sight Window).
 - Trunked Plants Plant selection of a mature trunk diameter 4" or less measured at 6" above the ground. Canopy or high borne foliage shall never be lower than 5' above the sight line datum. These selections shall be spaced no closer than 20'.

Trees:

Trees can be used with lawn; pavers; pavement; gravel, bark or wood chip beds; graund covers or other Department approved material.

The clear sight window must be in conformance with the 'WINDOW DETAIL' modified to attain the height requirements listed in 'Ground Covers' above. Tree size and spacing shall conform to the following tabular values:

	Speed (mph)													
Description		30	3	15	4	0	4	5	5	О	5	5	6	0
	(Inches)													
Diameter (Within Limits Of Sight Window)	>4 ≤11	>11 ≤18	>4 ≤11	>11 ≤18	>4 ≤//	>11 ≤18	>4 ≤11	>11 ≤18	>4 ≤11	>11 ≤18	>4 ≤//	>11 ≤18	>4 ≤//	>11 ≤18
	(Feet)													
Minimum Spacing (c. to c. Of Trunk)	22	9/	27	108	33	126	40	146	45	<i>165</i>	52	173	60	193

Sizes and spacings are based on the following conditions:

- (a) A single line of trees in the median parallel to but not necessarily colinear with the centerline,
- (b) A straight approaching mainline, within skew limits as described in No. 2 above.
- (c) I. Trees and palms ≤II" in diameter casting a vertical 6' wide shadow band on a vehicle entering at stop bar location when viewed by mainline driver beginning at distance 'd': see SHADOW DIAGRAM. Sheet 2.
- Sabal palms with diameters >II"to ≤ 18" spaced at intervals providing a 2 second full view of entering vehicle at stop bar location when viewed by mainline driver beginning at distance 'd'; see PERCEPTION DIAGRAM, Sheet 2.

For any other conditions the tree sizes, spacings and locations shall be detailed in the plans, see Design Note No. 5.

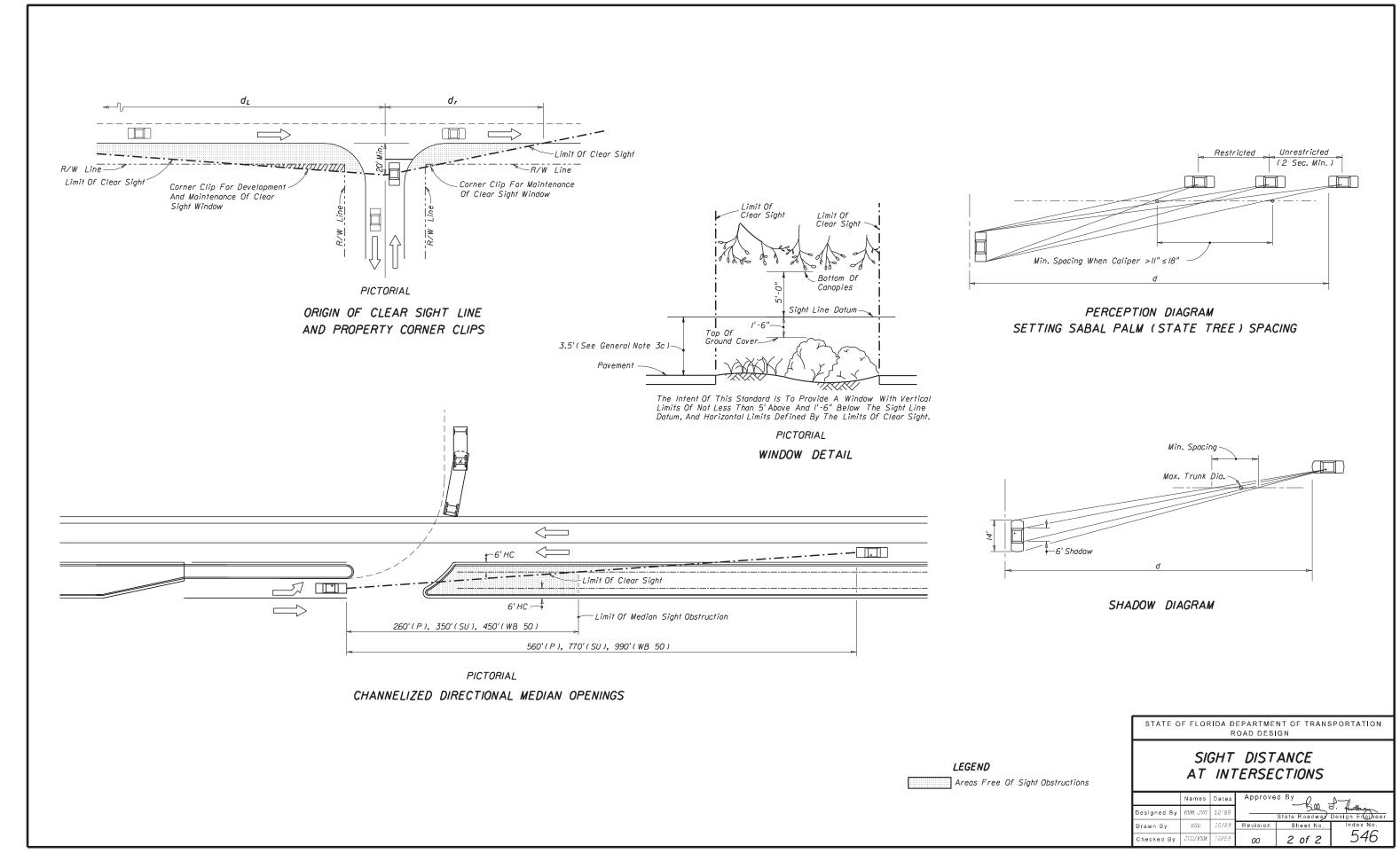
DESIGN NOTES

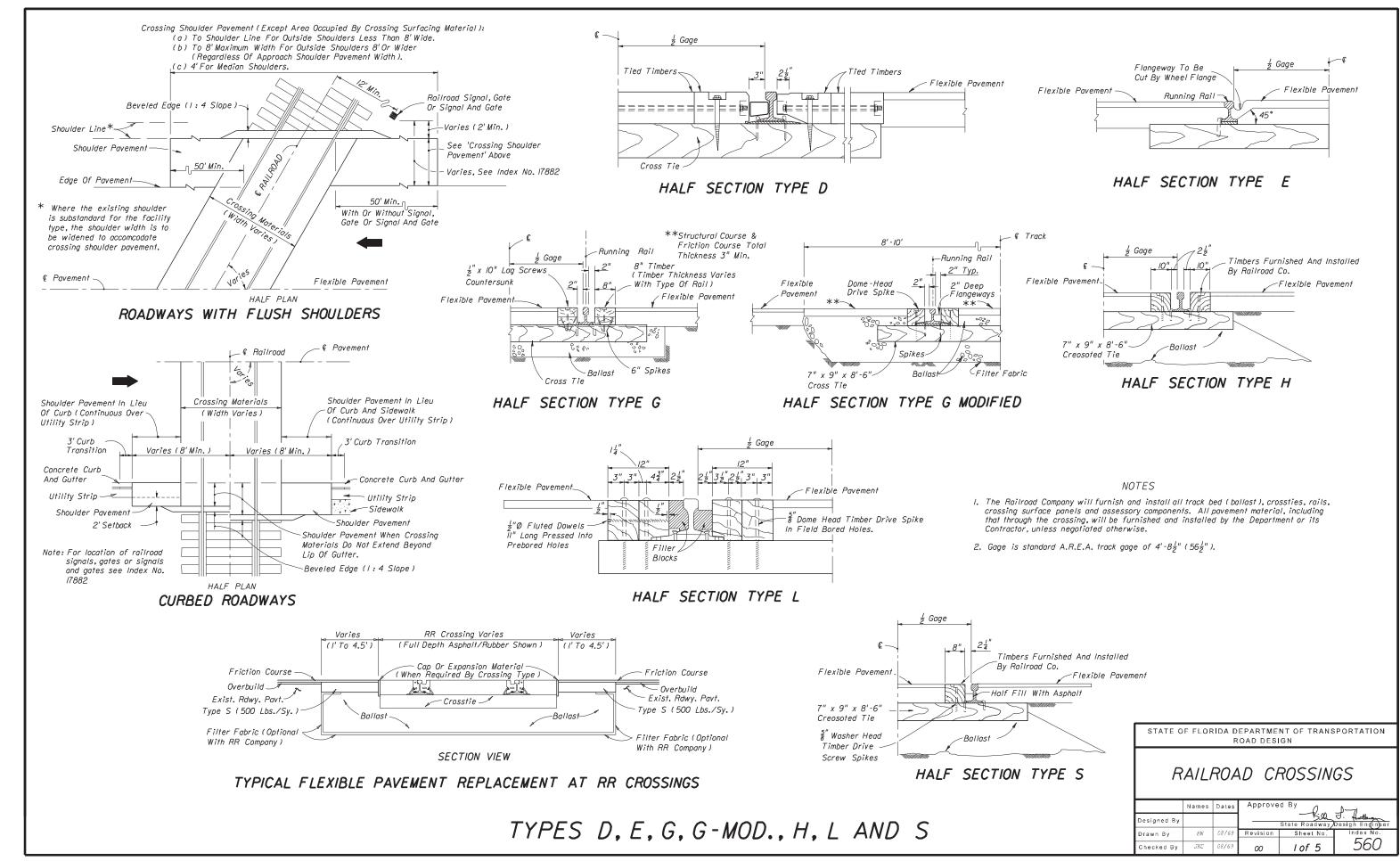
- I. The information shown on this index is intended solely for the purpose of clear sight development and maintenance at intersecting highways, roads and streets, and is not intended to be used to establish geometric design, speed control, signing, marking, lighting or signalization, or to establish roadway and roadside safety except as related to clear sight corridors. An analysis of sight distance shall be documented for all intersections.
- 2. Details are based on the AASHTO 'A Policy On Geometric Design Of Highways And Streets', Chapter IX, Cases III and IX, and Department practices for channelized median openings (left turns from major roadways).
- 3. The minimum driver eye setback of 20' from the edge of the traffic lane may be adjusted on any intersection leg only when justified by a documented, site specific field study of vehicle stopping position and driver eye position.
- 4. For SIGNALIZED INTERSECTIONS: Due to a variety of standard operational characteristics associated with signal controlled intersections, the sight distances based on Case III procedures should be available to the driver. Unanticipated vehicle conflicts at signalized intersections, such as violation of the signal, turns on red, malfunction of the signal, or use of the flashing red/yellow mode further substantiate the need for incorporation of Case III sight distances. If the proper sight distances can not be attained, other design features such as 'no right-on-red' may be necessary. Where landscaping is incorporated with construction or superimposed on existing facilities, the planting restrictions listed under the General Notes above are to be considered in the sight distance analysis.
- 5. Where curvature, superelevation, adverse split profiles or other conditions preclude the use of standard tree sizes and spacing, proof of view and shadowing restraints must be documented and the size and location of trees in medians detailed in the plans.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

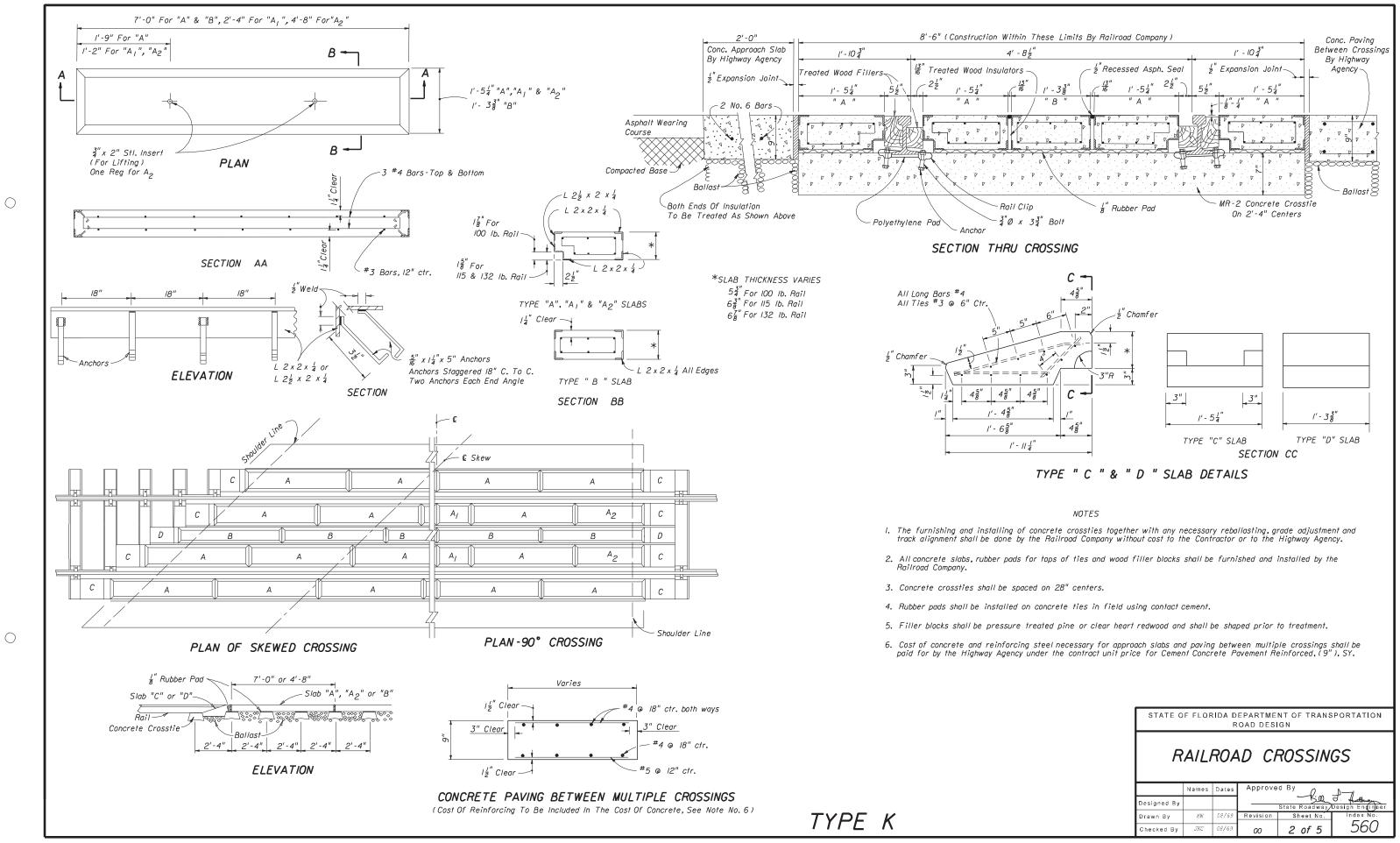
SIGHT DISTANCE AT INTERSECTIONS

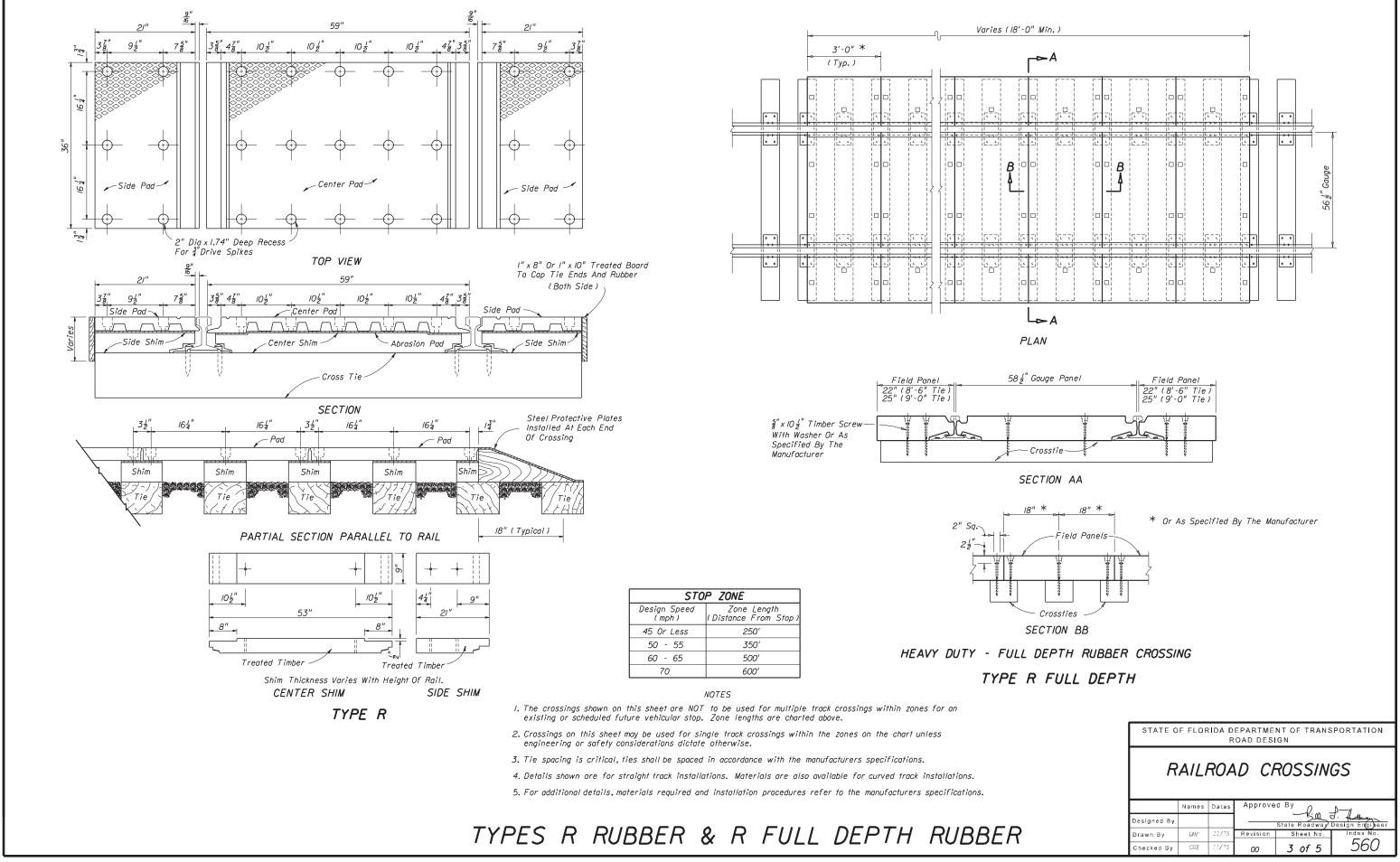
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Designed By	KNM/JVG	10/89	State Roadway Design Engile					
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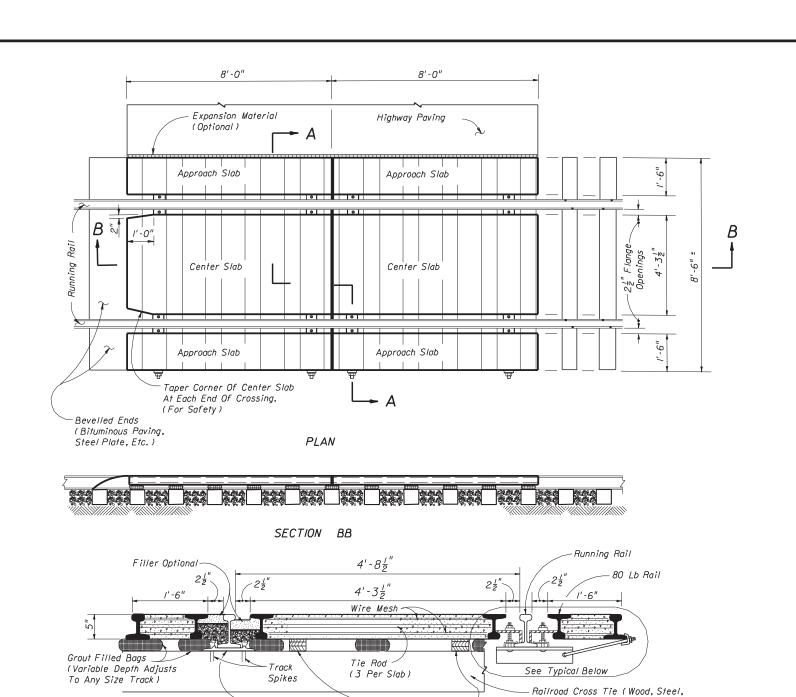




 \bigcirc







Single Or Double

Reactive Spring Washer

Insulator

Base Plate

Inside Hold - Down

Track Bolt

TYPICAL BOTH SIDES

Lock Nut

Rail Clip

Undertie Rod

 \bigcirc

Shoulder Tie Plate

SECTION AA

1. The reinforced concrete slabs are manufactured in 8'-0'' sections, 5'' in depth to fit all rail sections $5\frac{1}{4}''$ in height or heavier. Slabs are interchangeable and relocateable.

Concrete Or Bridge Decking)

2. Center slabs are one piece construction allowing for $2\frac{J}{2}''$ flange opening, 80 lb. rail is used to encase, armor and reinforce slabs and is held to gage with 3 tie rods per slabs.

3. Slabs are installed by a "flotation" process, supported on non-shrinkable, non-metallic grout positioned on the ties. Slabs can be placed on wood ties, concrete ties, steel ties, bridge decks or any other type of track support. No re-spacing of ties is necessary.

- 4. Slabs are secured to "running rails" with specially designed hardware. Insulation is to be provided for crossing in signal territory.
- 5. Curved slabs are fabricated to fit curved track to 22 degrees (262.04' radius). Special slabs are available for diamond crossings, turnouts, multiple tracks, bridge decks and rapid transit systems.
- 6. For additional details, materials required and installation procedures refer to the manufacturers specifications.
- 7. All asphalt will be installed in accordance with Index No. 513 and Section 300 of the Standard Specifications.

TYPE T

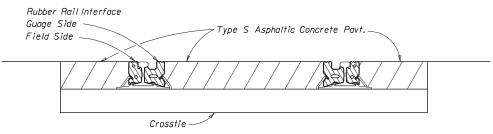
Temporary Shim Stacks-

(To Level Slab)

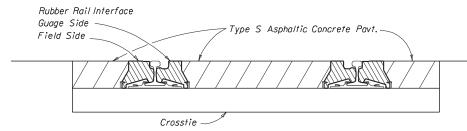
Lock Nut

Undertie Bracket

TYPES T & RS



ALTERNATE INTERFACE SECTION VIEW



ALTERNATE INTERFACE SECTION VIEW

NOTES

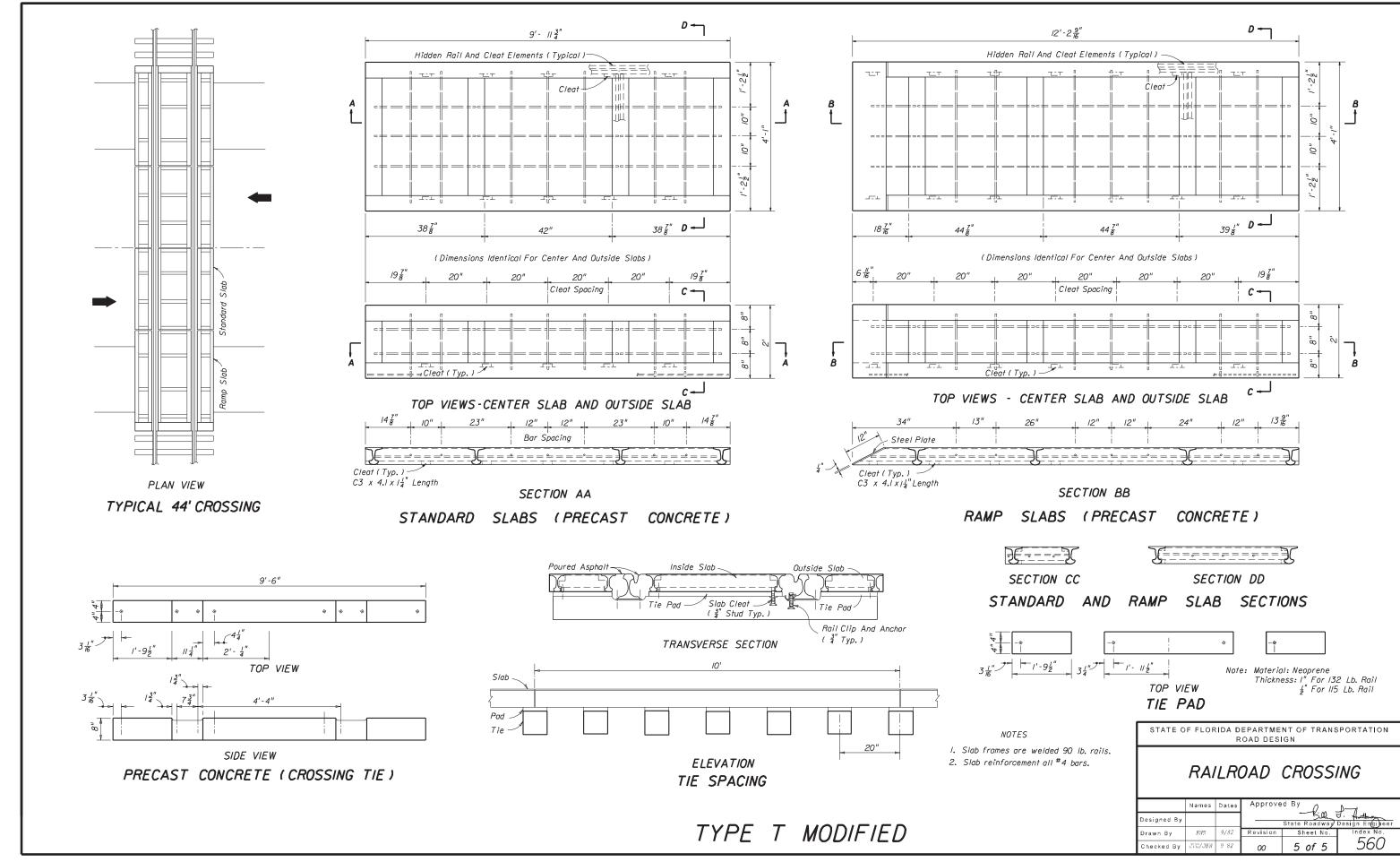
- Rubber rail interface systems are manufactured to fit various rails from II5# to I36#.
- The Railroad Company will furnish and install all crossing material except as specified in the agreement.
- 3. For additional details, methods required and installation procedures refer to the manufacturers specifications.

FULL DEPTH ASPHALT/RUBBER CROSSING TYPE RS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RAILROAD CROSSINGS

	Names	Dates	Approve	d By	N
Designed By				2000	Design Engineer
Drawn By	LMF.	02/77	Revision	Sheet No.	Index No.
Checked By	GSB	02/77	00	4 of 5	560



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PREFACE

All projects and works on highways, roads and streets shall have a traffic control plan. All work shall be executed under the established plan and Department approved procedures. This index contains information specific to the Federal and State guidelines and standards for the preparation of traffic control plans and for the execution of traffic control in work zones, for construction and maintenance operations and utility work on highways, roads and streets.

Index 600 provides Department policy and standards. Changes are only to be made thru Department approved procedures. Indexes 601 thru 665 provide typical application for various situations. Modification can be made to these Indexes as long as the changes comply with the M.U.T.C.D. and Department standards.

The sign spacings shown on the Indexes are typical (recommended) distances. These distances may be increased or decreased based on field conditions, in order to avoid conflicts or to improve site specific traffic controls.

MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES

The Florida Department of Transportation has adopted the "Manual On Uniform Traffic Control Devices For Streets And Highways" (MUTCD) and subsequent revisions and addendums, as published by the U.S. Department of Transportation, Federal Highway Administration, for mandatory use on the State Maintained Highway System whenever there exists the need for construction, maintenance operations or utility work.

ABBREVIATIONS

Abbreviations assigned to the 600 series Roadway Design Standards and applicable to traffic control plans, unless otherwise identified in the plans, are as follows:

TCPTraffic control plan(s)

MUTCD 'Manual On Uniform Traffic Control Devices For Streets And Hiahways'

TCZTraffic control through work zones

Taper length, buffer length or taper length plus buffer space

Width of taper transition in feet, i.e., lateral offset Posted speed or off-peak 85 percentile speed (mph) S

RPM Raised reflectorized pavement marker

Truck mounted attenuator TMA

COMM Traffic Control Standards Committee

VMS Variable Message Sign

Designates Maintenance Of Traffic Signs MOT

FDOT Florida Dept. Of Transportation DTOE District Traffic Operations Engineer

Radius

Orange Flag For TCZ Signs

 $\langle Q \rangle$ Type B Light For TCZ Signs

SYMBOLS

The symbols shown are found in the Traffic Control Zone Cell Library (TCZ.cel) on the CADD system.

Symbols assigned to the 600 series Roadway Design Standards and applicable to traffic control plans, unless otherwise identified in the plans, are as follows:

Work Area, Hazard Or Work Phase (Any pattern within a boundary)

 $lap{>}{\sim}$ Sign With 18" x 18" (Min.) Orange Flag And Type B Light

Type I Or Type II Barricade Or Vertical Panel Or Drum

Type I Or Type II Barricade Or Vertical Panel Or Drum (With Flashing Light At Night Only)

Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light

Type I Or Type II Barricade Or Vertical Panel Or Cone Or Tubular Marker Or Drum

Cone Or Tubular Marker

Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum

Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum (With Flashing

Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum (With Steady Burning Light)

Type III Barricade

Type III Barricade (With Flashing Light)

Type III Barricade (With Steady Burning Light)

₩ork Zone Sign

□ Traffic Signal

Advance Warning Arrow Panel

Portable Signal

C. C. Crash Cushion

Stop Bar

₩ ₩ ₩ Work Vehicle With Flashing Beacon

Shadow (S) Or Advance Warning (AW) Vehicle Shadow (5) Or Advance Warning Car, Volume Sign With Advance Warning Arrow Panel And Warning Sign

A Truck Mounted Attenuator (TMA)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES GENERAL INFORMATION FOR TRAFFIC CONTROL THROUGH WORK ZONES

Designed By rawn Bv 600 hecked By 00 1 of 11

DEFINITIONS

Regulatory Speed (In Work Zones)

The maximum permitted travel speed posted for the work zone as indicated by the regulatory speed limit signs. The work zone speed must be shown or noted in the plans. This speed should be used as the minimum design speed to determine runout lengths, departure rates, flare rates, lengths of need, clear widths, taper lengths, crash cushion requirements, marker spacings, superelevation and other similar features.

Advisory Speed

The maximum recommended travel speed through a curve or a hazardous area.

Travel Way

The intended path for vehicular traffic through or around obstructions in construction, maintenance, utility and other work zones on highways, roads and streets. For traffic control through work zones, travel way includes auxiliary lanes, shoulders and any other permanent or temporary surface intended for the path of vehicular traffic.

Detour

A detour is the redirection of traffic onto another roadway to bypass the temporary traffic control zone. A lane shift is the redirection of traffic onto a different section of the permanent pavement. A diversion is the redirection of traffic onto a temporary roadway, usually adjacent to the permanent roadway and within the limits of the right-of-way.

Above Ground Hazard

An above ground hazard is any object, material or equipment other than traffic control devices that encroaches upon the travel way or that is located within the clear zone which does not meet the Departments safety criteria, i.e., anything that is greater than 4" in height and is firm and unyielding or doesn't meet breakaway requirements.

EXTENDED DISTANCE ADVANCE WARNING SIGNS

Advance Warning Signs shall be used at extended distance of one-half mile or more when limited sight distance or the nature of the obstruction may require a motorist to bring their vehicle to a stop. Extended distance Advanced Warning Signs may be required on any type roadway, but particularly be considered on multi-lane divided highways where vehicle speed is generally in the higher range (45 M.P.H. or more).

REGULATORY SPEEDS IN WORK ZONES

Traffic Control Plans (TCPs) for all projects must include specific regulatory speeds for each phase of work. This can either be the posted speed or a reduced speed. The speed shall be noted in the TCPs; this includes indicating the existing speed if no reduction is to be made. Regulatory speeds are to be uniformly established through each phase.

In general, the regulatory speed should be established to route vehicles safely through the work zone as close to normal highway speed as possible. The regulatory speed should not be reduced more than IO mph below the posted speed and never below the minimum statutory speed for the class of facility. When a speed reduction greater than IO mph is imposed, the reduction is to be done in IO mph per 500' increments.

Temporary regulatory speed signs shall be removed as soon as the conditions requiring the reduced speed no longer exist. Once the work zone regulatory speeds are removed, the regulatory speed existing prior to construction will automatically go back into effect unless new speed limit signing is provided for in the plans.

On projects with interspaced work activities, speed reductions should be located in proximity to those activities which merit a reduced speed, and not "blanketed" for the entire project. At the departure of such activities, the normal highway speed should be posted to give the motorist notice that normal speed can be resumed.

If the existing regulatory speed is to be used, consideration should be given to supplementing the existing signs when the construction work zone is between existing regulatory speed signs. For projects where the reduced speed conditions exist for greater than I mile in rural areas (non-interstate) and on rural or urban interstate, additional regulatory speed signs are to be placed at no more than I mile intervals. Engineering judgement should be used in placement of the additional signs. Locating these signs beyond ramp entrances and beyond major intersections are examples of proper placement. For urban situations (non-interstate), additional speed signs are to be placed at a maximum of 1000' apart.

When field conditions warrant speed reductions different from those shown in the TCP the contractor may submit to the project engineer for approval by the Department, a signed and sealed study to justify the need for further reducing the posted speed, or, the engineer may request the District Traffic Operations Engineer (DTOE) to investigate the need. It will not be necessary for the DTOE to issue regulations for regulatory speeds in work zones due to the revised provisions of F.S. 316.0745(2) (b). Advisory Speed plates will be used at the option of the field engineer for temporary use while processing a request to change the regulatory speed specified in the plans when deemed necessary. Advisory speed plates cannot be used alone but must be placed below the construction warning sign for which the advisory speed is required.

For additional information refer to the FDOT Roadway Plans Preparation Manual, Volume I, Chapter IO.

ADJOINING AND/OR OVERLAPPING WORK ZONE SIGNING

Adjoining work zones may not have sufficient spacing for standard placement of signs and other traffic control devices in their advance warning areas or in some cases other areas within their traffic control zones. Where such restraints or conflicts occur or are likely to occur, one of the following methods will be employed to avoid conflicts and prevent conditions that could lead to misunderstanding on the part of the traveling public as to the intended travel way by the traffic control procedure applied:

- (a) For scheduled projects the engineer in responsible charge of project design will resolve anticipated work zone conflicts during the development of the project traffic control plan. This may entail revision of plans on preceding projects and coordination of plans on concurrent projects.
- (b) Unanticipated conflicts arising between adjoining in progress highway construction projects will be resolved by the Resident Engineer for projects under his residency, and, by the District Construction Engineer for in progress projects under adjoining residencies.
- (c) The District Maintenance Engineer will resolve anticipated and occurring conflicts under the following work zone conditions.
 - I. Within scheduled maintenance operations.
 - 2. Between scheduled maintenance operations, maintenance construction, permitted works and/or in progress highway construction projects.
- (d) The Unit Maintenance Engineer will resolve conflicts that occur within routine maintenance works; between routine maintenance work, unscheduled work and/or permitted work; and, between unit controlled maintenance works and highway construction projects.

INTERSECTING ROAD SIGNING

Signing for the control of traffic entering and leaving work zones by way of intersecting highways, roads and streets shall be adequate to make drivers aware of work zone conditions. Under no condition will intersecting leg signing be less than a ROAD WORK AHEAD sign, including light and flag, for approaching vehicles.

SIGNALS

Existing traffic signal operations that require modification in order to carry out work zone traffic control shall be included in the TCP and be approved by the District Traffic Operations Engineer. The need for temporary signal loops or other methods of actuation shall be determined by the District Traffic Operations Engineer and the designer and included in the TCP.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

GENERAL INFORMATION FOR TRAFFIC CONTROL THROUGH WORK ZONES

	Names	Dates	Approve		To Oa M
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Drawn By		12/87	Revision	Sheet No.	Index No.
Checked By		12/87	00	2 of 11	600

CHANNELIZING AND LIGHTING DEVICES

Channelizing and lighting devices for work zone traffic control shall be as prescribed in Part VI of the MUTCD, subject to supplemental revisions provided in the contract documents.

Primary work zone traffic control devices are shown on Sheet 8 for the purpose of ready identification. Approved devices are listed on the Departments Qualified Product List.

DROPOFFS IN WORK ZONES

Acceptable warning and barrier devices for traffic control at dropoffs in work areas are detailed on Sheet 6. Unless otherwise specified in the plans, the contractor may use any of the barrier types shown in note 3 on sheet 6.

WARNING LIGHTS

Warning lights shall be in accordance with Section 6E-5 of the MUTCD except for the application limitations and methods of payment stipulated below:

Flashina

Type A Low Intensity Flashing Warning Lights are to be mounted on barricades, drums, vertical panels or advance warning signs (except as noted below) and are intended to continually warn drivers that they are approaching or proceeding in a hazardous area. Flashing lights shall not be used to delineate the intended path of travel, and not placed with spacings that will form a continuous line to the drivers eye. The Type A light will be used to mark obstructions that are located adjacent to or in the intended travel way. Type A lights shall not be used in conjunction with the first advance warning sign nor the second such sign when used.

Type B High Intensity Flashing Warning Lights shall be mounted on the first advanced warning sign and on the first and second advanced warning sign where two or more signs are used; this applies to all approaches to any work zone.

Steady-Burn

Type C Steady-Burn Lights are to be mounted on barricades, drums, concrete barrier walls or vertical panels and used in combination with those devices to delineate the travel way on lane closures, lane changes, detour curves and other similar conditions. Steady-burn lights are intended to be placed in a line to delineate the traveled way through and around obstructions in the transition, buffer, work and termination areas of the traffic control zone. Their intended purpose is not for warning drivers that they are approaching or proceeding through a hazardous area.

SIGHT DISTANCE TO DELINEATION DEVICES

Transition tapers should be obvious to drivers. If restricted sight distance is a problem (e.g., a sharp vertical or horizontal curve), the taper should begin well in advance of the view obstruction. The beginning of tapers should not be hidden behind curves.

CHANNELIZING AND LIGHTING DEVICE CONSISTENCY

Barricades, vertical panels, cones, tubular markers and drums shall not be intermixed within either the lateral transition or within the tangent alignment.

FLAGGING OPERATIONS

When operations are such that signs, signals and barricades do not provide adequate protection on or adjacent to a highway or street, flaggers and / or other appropriate traffic control shall be provided. Flagger station(s) shall be located far enough ahead of the work space so that approaching traffic has sufficient distance to stop before entering the work space.

Stop /Slow Paddles are the primary hand-signaling device. Flag use is limited to Immediate Emergencies, Intersections, and when working on centerline or shared left turn lanes where two (2) flagmen are required and there is opposing traffic in the adjacent traffic lanes. Where Flagmen are used, a FLAGMAN symbol or legend sign must replace the WORKERS symbol or legend sign.

NIGHTTIME FLAGGING

Nighttime flagging will require proper illumination of the flagger. A well lighted flagging station and/or a reflectorized paddle or reflectorized flag, plus a flashlight, lantern or other lighted signal that will display a red warning light shall be used. Lights, reflectorized paddles, reflectorized flags and reflectorized vests, shirts or jackets approved by the Department must be used to flag traffic at night. The STOP face of paddles shall be reflectorized red with white reflectorized letters and border, and the SLOW face shall be reflectorized orange with black letters and border. Flagger vests, shirts or jackets shall be reflectorized orange.

The flagger must be clearly visible to approaching traffic for a distance sufficient to permit proper response by the motorist to the flagging instructions, and to permit traffic to reduce speed or to stop as required before entering the work site. Flaggers shall be positioned to maintain maximum color contrast between the flaggers reflective garments and equipment and the work area background.

REMOVING PAVEMENT MARKINGS

Existing pavement markings that conflict with temporary work zone delineation shall be removed by any method approved by the Engineer, where operations exceed one daylight period; however, painting over existing pavement markings will not be permitted. Full pavement width overlays of either asphalt concrete SP 9.5 or FC-6 is a positive means to achieve obliteration.

SUPERELEVATION

Horizontal curves constructed in conjunction with work zone traffic control should have the required superelevation applied to the design radii. Under conditions where normal cross slope controls curvature, the minimum radii that can be applied are listed in the table below.

MINIMUM RADII FOR NORMAL CROSS SLOPES						
DETOUR DESIGN SPEED	MINIMUM RADIUS R					
MPH 65	feet 3/30					
60	2400					
55 50	1840 1390					
45	1080					
<u>40</u> 35	820 610					
30	430					
Superelevate When	Smaller Radii Used					

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

GENERAL INFORMATION FOR
TRAFFIC CONTROL THROUGH WORK ZONES

	Names	Dates	Approve	' / /	De Orill
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Drawn By		12/87	Revision	Sheet No.	Index No.
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END ROAD WORK SIGNS

The END ROAD WORK sign (G20-2A) should be erected approximately 500 feet beyond the end of a construction or maintenance project, unless other distance called for in the plans. Where other Construction or Maintenance Operations occur within I mile this sign should be omitted and signing coordinated in accordance with Index No. 600, ADJOINING AND/OR OVERLAPPING WORK ZONE SIGNING.

DETOURS

Detours should be signed clearly over their entire length so that motorists can easily determine how to return to the original roadway. The WI-4R, MOT-2, and MOT-3 warning signs are to be used for the advanced warning for a lane shift. A diversion should be signed as a lane shift.

VARIABLE MESSAGE SIGNS (VMS)

The VMS can be used to:

- (I) Supplement standard signing in construction/maintenance work zones.
- (2) Reinforce static advance warning messages.
- (3) Provide motorists with updated guidance information.

The message should be visible and legible at a minimum distance of 900 feet. All messages should be cycled so that two message cycles are displayed to a driver while approaching the sign from 900 feet at 55 mph.

VMS should be placed approx. 500 to 800 feet in advance of the work zone conflicts or 1.5 to 2 miles in advance of complex traffic control schemes which require new and/or unusual traffic maneuvers.

If VMS are to be used at night, the intensity of the flashers shall be reduced during darkness when lower intensities are desirable.

For additional information refer to the FDOT Roadway Plans Preparation Manual, Volume I, Chapter IO.

ROADSIDE BARRIERS

When connecting temporary concrete barrier wall to guardrail the connection shall be made in accordance with Index No. 410.
All guardrail end anchorages to be included in the cost of Temporary Guardrail.

ABOVE GROUND HAZARD

Above ground hazards (see definitions) are to be considered work areas during working hours and treated with appropriate work zone traffic control procedures. During non-working hours, all objects, materials and equipment that constitute an above ground hazard must be stored/placed outside the travel way and clear zone or be shielded by a barrier or crash cushion.

For above ground hazards within a work zone the clear zone required should be based on the regulatory speed posted during construction.

WORK ZONE SIGN SUPPORTS

All post mounted Work Zone signs shall be installed on either round aluminum or steel channel post as specified in the table below.

SUPPORTS FOR MAINTENANCE OF TRAFFIC SIGNS								
SIGN SIZE	SIGN BRACKET	ROUND ALUMINUM	DEPTH IN GROUND	STEEL CHANNEL	DEPTH IN GROUND			
24" x 36"	2-I	NPS 2.0" $x \frac{1}{8}$ "	2'-0"	2.5 lb F/M*	3'-0"			
48" x 48" DIAMOND	2-I& I-II	NPS 3.5" x 3/6"	3'-4"	**	3'-0"			
60" x 48"	3 <i>-I</i>	NPS 3.5" x 3/16"	3'-4"	**	3'-0"			
24" x 30"	2-I	NPS 2.0" $x \frac{1}{8}$ "	2'-0"	2.5 1b F/M*	3'-0"			
48" x 48"	2-Ⅱ	NPS 3.0" x ½"	2'-6"	**	3'-0"			
60" x 24"	3-I	NPS 3.0" $x \frac{1}{8}$ "	2'-6"	3.0 lb F/M*	3'-0"			
60" x 36"	3-I	NPS 3.5" $x \frac{3}{16}$ "	3'-4"	4.0 lb F/M*	3'-0"			

* F/M Indicates Type F or Type M

** Requires two 3 lb/ft steel channel (F/M) at 2'-6" center to center. All sign brackets shall be Type I. The total number of brackets shall be per post as tabulated, except the "Diamond" sign which shall use two Type I brackets per post.

The 4 lb/ft steel channel shall be installed with approved breakaway bases.

Refer to Design Standard II860, Sheet 3, for round aluminum sign bracket details, and II865 Sheet 2 for steel channel breakaway bases, and notes.

CLEAR ZONE WIDTHS

The term 'clear zone' describes the unobstructed relatively flat area, impacted by construction, extending outward from the edge of the travel lane. The table below gives clear zone widths in work zones for medians and roadside conditions other than for roadside canals; where roadside canals are present, clear zone widths are to conform with the distances to canals as described in Volume I Ch. 4, Sec. 4.2 and Exhibit 4-A and 4-B of the Plans Preparation Manual.

CLEAR ZONE WIDTH	S FOR WORK ZONES
WORK ZONE SPEED (MPH)	WIDTHS (feet)
60 - 70	30
55	24
45 - 50	18
30 - 40	14
ALL SPEEDS CURB & GUTTER	4' BEHIND FACE OF CURB

SIGN MATERIALS

Mesh signs may be used only for Daylight Operations as noted in the standards. Type B Lights and Orange Flags are not required.

Vinyl signs may be used for Day or Night Operations not to exceed 12 hours except as noted in the standards. Type B Lights and Orange Flags are not required.

All signs shall be post mounted if operation exceeds 12 hours except as noted in the standards.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

GENERAL INFORMATION FOR

TRAFFIC CONTROL THROUGH WORK ZONES

	Names	Dates	Approve	d By	To Oil
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SURVEY WORK ZONES

The SURVEY CREW AHEAD symbol or legend sign shall be the principal Advance Warning Sign used for Traffic Control Through Survey Work Zones and may replace the ROAD WORK AHEAD sign when lane closures occur, at the discretion of the Party Chief. Type B Light or dual orange flags shall be used at all times to enhance the SURVEY CREW AHEAD sign, even with mesh signs.

When Traffic Control Through Work Zones is being used for Survey purposes only, the END ROAD WORK sign as called for on certain 600 Series Indexes should be omitted.

Survey Between Active Traffic Lanes or Shared Left Turn Lanes

The following provisions apply to Main Roadway Traffic Control Work Zones. These provisions must be adjusted by the Party Chief to fit roadway and traffic conditions when the Survey Work Zone includes Intersections.

- (A) A STAY IN YOUR LANE (MOT-I) sign shall be added to the Advance Warning Sign sequence as the second most immediate sign from the work area.
- (B) Elevation Surveys-Cones may be used at the discretion of the Party Chief to protect prism holder and flagger(s). Cones, if used, may be placed at up to 50' intervals along the break line throughout the work zone.
- (C) Horizontal Control-With traffic flow in the same direction, cones shall be used to protect the backsight tripod and/or instrument. Cones shall be placed at the equipment, and up to 50' intervals for at least 200' towards the flow of traffic.
- (D) Horizontal Control-With traffic flow in opposite directions, cones shall be used to protect the backsight tripod and/or instrument. Cones shall be placed at the equipment, and up to 50' intervals for at least 200' in both directions towards the flow of traffic.

PEDESTRIANS AND BICYCLIST

When an existing pedestrian way or bicycle way is located within a traffic control work zone, accommodation must be maintained and include provision for the disabled.

RAILROADS

Railroad crossings affected by a construction project should be evaluated for traffic controls to reduce queuing on the tracks. The evaluation should include as a minimum: traffic volumes, distance from the tracks to the intersections, lane closure or taper locations, signal timing, etc.

SIGN COVERING AND INTERMITTENT WORK STOPPAGE SIGNING

Existing signs that conflict with temporary work zone signing shall be removed or covered as approved by the Engineer. Traffic control signs that require covers when no work is being performed in a work area shall be fully covered with a durable opaque sheet material.

Plastic film and woven fabrics including burlan will not be permitted.

Plastic film and woven fabrics including burlap will not be permitted. Covering of only the legend or symbol will not be permitted.

Reflective coverings will not be permitted.

Hinged signs designed to cover when folded and sign blanks will be permitted. Covers, blanks, hinged panels and intermittent work stoppage shields and plaques are incidental to work operation signs and are not to be paid for separately.

LANE WIDTHS

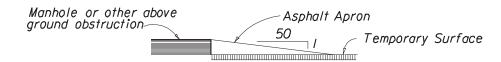
Lane widths of through roadways should be maintained through work zone travel ways wherever practical. The minimum widths for work zone travel lanes shall be as follows: IO' for Interstate with at least one I2'-O" lane provided in each direction, unless formally excepted by the Federal Highway Administration; IO' for freeways; and 9' for all other facilities.

LENGTH OF ROAD WORK SIGN

The length of road work sign (G20-I) bearing the legend ROAD WORK NEXT____ MILES is required for all projects of more than 2 miles in length. The number of miles entered should be rounded up to the nearest mile. The sign shall be located at begin construction points.

MANHOLES/CROSSWALKS

Manholes extending I" or more above the travel lane and crosswalks having an uneven surface greater than $\frac{1}{2}$ " shall have a temporary asphalt apron constructed as shown in the diagram below.



The apron is to be removed prior to constructing the next lift of asphalt The cost of the temporary asphalt shall be included in the Contract Unit Price for Maintenance of Traffic, L.S.

TRUCK MOUNTED ATTENUATORS

Truck-mounted attenuators (TMA) can be used for moving operations and short-term stationary operations. For moving operations, see Index No. 627. For short term, stationary operations, see Part VI of the MUTCD.

SPEEDING FINES DOUBLED WHEN WORKERS PRESENT SIGN

The SPEEDING FINES DOUBLED WHEN WORKERS PRESENT sign should be installed on all projects.

The placement should be 500 ft beyond the ROAD WORK AHEAD sign or midway to the next sign whichever is less.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

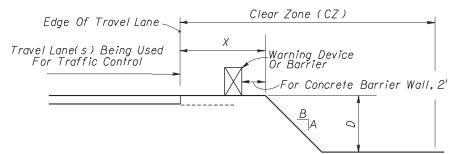
GENERAL INFORMATION FOR

TRAFFIC CONTROL THROUGH WORK ZONES

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DROPOFF CONDITION

- I. A dropoff is defined as a drop in elevation, parallel to the adjacent travel lanes, greater than 3" with slopes (A:B) steeper than I:4. When dropoffs occur within the clear zone during construction or maintenance activities, protection devices will be required, see chart.
- 2. Distance X is to be the maximum practical under project conditions.
- 3. Distance from the travel lane to the barrier or warning device should be maximum practical for project conditions.
- 4. Any dropoff condition that is created and restored within the same work period will not be subject to the use of barriers; however, warning devices will be required.
- 5. When permanent curb heights are \geq 6", no warning device will be required. For curb heights < 6", see chart.



DROPOFF PROTECTION REQUIREMENTS ALL SPEEDS NO CURB AND GUTTER						
X (ft)	D (in)	Device Required				
0 - CZ	≤ 3	Sign W8-9AS				
0-12	>3	Barrier				
12-CZ	>3 to≤5	Warning Device				
0 - CZ	>5	Barrier				

For Clear Zone widths, see Index No. 600 sheet 4.

DROPOFF NOTES

- I. These conditions and treatments can be applied only in work areas that fall within a properly signed work zone.
- 2. The following are defined as acceptable warning devices:
 - a. Vertical Panel
 - b. Type I Or Type II Barricades
 - c. Drum

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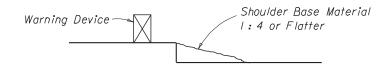
- d. Cone (where allowed)
- e. Tubular Marker (where allowed)
- 3. Where a barrier is specified any of the types below may be used as shown in the plans:
 - a. Concrete temporary barrier wall;
 - b. Temporary guardrail and end anchorages;
 - c. Temporary Curb;
 - d. Temporary water filled barriers.

- 4. Warning device spacing shall be as follows:
 - A. On Taper

Maximum spacing between cones and tubular markers shall be 25'. Maximum spacing between Type I or Type II barricades or vertical panels or drums shall be based on the speed limit as follows: 15' up to 25 MPH; 30' for 30 - 40 MPH; 50' for 45 MPH and greater.

B. On Alignments

Maximum spacing between cones or tubular markers shall be 25'. and for Type I or Type II barricades, vertical panels or drums is 50' on center for the first 250'; thereafter, cones or tubular markers at 50' on center and Type I or Type II barricades drums or vertical panels at 100' on center.

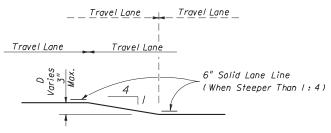


NOTES

- I. The contractor may use shoulder treatment in lieu of barrier. Warning devices are required.
- 2. Daily inspections shall be conducted to assure that no erosion, excessive slopes, rutting, or other adverse conditions exist.

 Any deficiencies shall be repaired immediately.
- 3. Compensation for the placement and removal of the material required for the shoulder treatment shall be included in the cost for Maintenance Of Traffic, LS. Use of shoulder treatment in lieu of a barrier is not eligible for VECP consideration.

SHOULDER TREATMENT



NOTES

- I. This treatment applies to resurfacing or milling operations between adjacent travel lanes.
- 2. Whenever there is a difference in elevation between adjacent travel lanes, the W8-9A sign with "UNEVEN PAVEMENT" plaque is required at intervals of $\frac{1}{2}$ mile maximum.
- 3. If D is $l_2^{l_1}$ or less, no treatment is required.
- 4. Treatment allowed only when D is 3" or less.
- 5. If the slope is steeper than I: 4 (not to be steeper than I:I), the R4-I and MOT-I signs shall be used as a supplement to the W8-9A; this condition should never exceed 3 miles in length.

TRAVEL LANE TREATMENT FOR MILLING OR RESURFACING

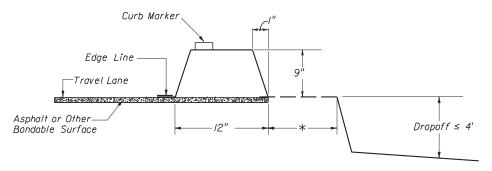
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

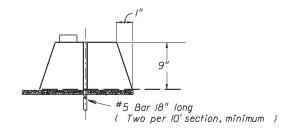
GENERAL INFORMATION FOR TRAFFIC CONTROL THROUGH WORK ZONES

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DROPOFFS IN WORK ZONES



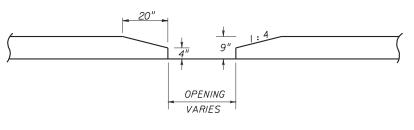
TEMPORARY CURB DETAIL



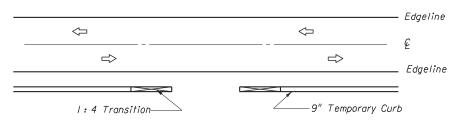
PINNING DETAIL

* 12" (or more) is desirable in order to enhance/improve stability.

However, it is recognized that there may be cases where 12" (or more)
is not feasible or obtainable. In these instances, engineering judgement must be
used to balance this offset distance with the depth of dropoff, soil type, etc.



ELEVATION



PLAN
TEMPORARY CURB OPENINGS

TEMPORARY CURB

- I. Application: Temporary curb shall not be used on facilities with posted speeds greater than 45 mph or dropoffs greater than 4'deep. It shall not be used on Interstate or limited access facilities.
- 2. Edgelines shall be provided in accordance with the traffic striping specifications, including reflective beads. The face of the curb shall also be painted (white or yellow as appropriate). A Curb Marker shall be placed on the temporary curb every IO'(Colorless when curb is on the right side of the lane, and amber when the curb is on the left side of the lane).
- 3. The temporary asphalt curb is to be bonded to the surface by use of a tack coat. It is important that the curb adhere to the surface in order to provide the strength necessary to redirect errant vehicles. Concrete curb and curb of other approved materials shall be pinned to a paved surface as shown in detail.
- 4. When temporary curb is call for in the plans the contractor has the option to construct temporary curb of asphalt, Class I concrete, or other Department approved material.
- 5. When concrete is used to construct temporary curb, $\frac{1}{2}$ " open joints shall be constructed every 10' in order to control cracking.
- 6. Drainage needs must be addressed when using temporary curb. If driveways or other accesses are not frequent enough to allow for water runoff, the designer may need to specify the need for "drainage slots" at an appropriate spacing based on grades, number of lanes, etc. Typically, a drainage slot should be 12" wide (a break in the curb) at 50' spacings.
- 7. At openings such as driveways and business accesses, the temporary curb should be transitioned in height from 4" up to 9" at a 1:4 slope in order to eliminate a potential hazard at the end points.
- 8. Temporary curb shall be paid for under the contract unit price for Temporary Curb, LF, and will include all materials (including Curb Markers) and work necessary to construct, maintain and remove the temporary curb. Any damage to existing powement caused by the removal of temporary curb shall be satisfactorily repaired and the cost of such repairs are to be included in the cost of the temporary curb.

TEMPORARY CURB

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

GENERAL INFORMATION FOR TRAFFIC CONTROL THROUGH WORK ZONES

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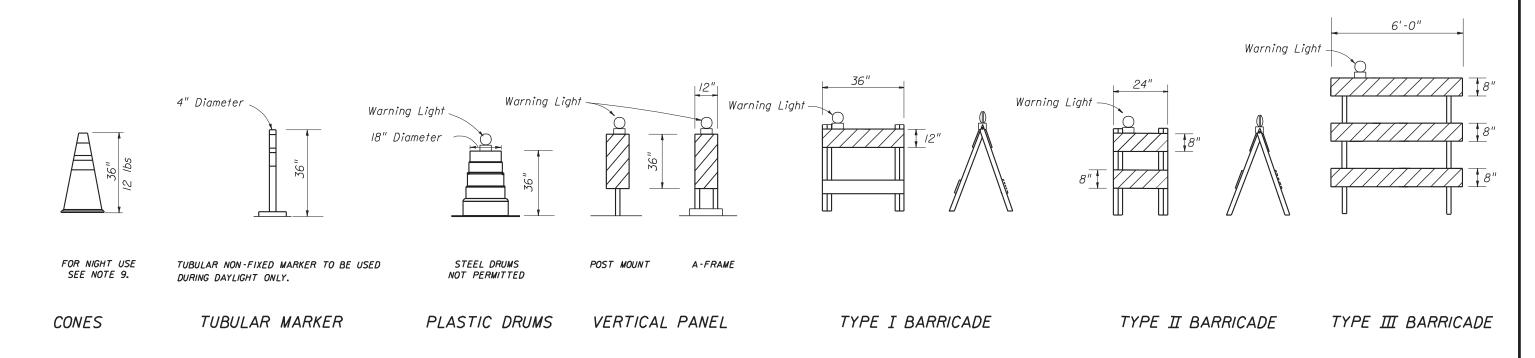
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Roadway Design Engineer

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CHANNELIZING AND LIGHTING DEVICE NOTES

0 O \hat{O} O20000 (0,0)PASS LEFT PASS RIGHT WARNING Minimum Required Lamps Additional Lamps Allowed PASS EITHER SIDE

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MODES
ADVANCE WARNING ARROW PANELS

- I. Only approved traffic control devices may be used.
- 2. The FDOT approval number shall be engraved on the device at a convenient and readily visible location. Where engraving is not practical a water-resistant type label may be used.
- 3. The details shown on this sheet are for the following purposes: (a) For ease of identification and (b) To provide information that supplements or supercedes that provided by the MUTCD.
- 4. The Type III Barricade shall have a unit length of 6'-0" only. When barricades of greater lengths are required those lengths shall be in multiples of the 6'-0" unit. Signs used in conjunction with Type III Barricades may be mounted on or above the Barricade. These Signs should not cover more than 50 percent of the top two rails or 33 percent of the total area of the three rails.
- 5. During hours of darkness, warning lights shall be used on drums, vertical panels, Type I, Type II and Type III barricades in accordance with 'Warning Lights' Sheet 3.

- 6. Ballast shall not be placed on top rails or any striped rails or higher than 13" above the driving surface.
- 7. For rails less than 3'-0" long, 4" stripes shall be used.
- 8. When Advance Warning Arrow Panels are used at night, the intensity of the flashers shall be reduced during darkness when lower intensities are desirable.
- 9. Cones Shall:
 - I. Be used only in work zones where workers are present.
 - 2. Not exceed I mile in length of use at any one time nor exceed a 12 hour work period.
 - 3. Have as a minimum, one designated person for the purpose of continuous monitoring and maintenance of cones during lane closures.
 - 4. Be reflectorized as per the MUTCD with Department approved reflective collars when used at night.
- IO. The splicing of sheeting is not permitted on either channelizing devices or MOT signs.

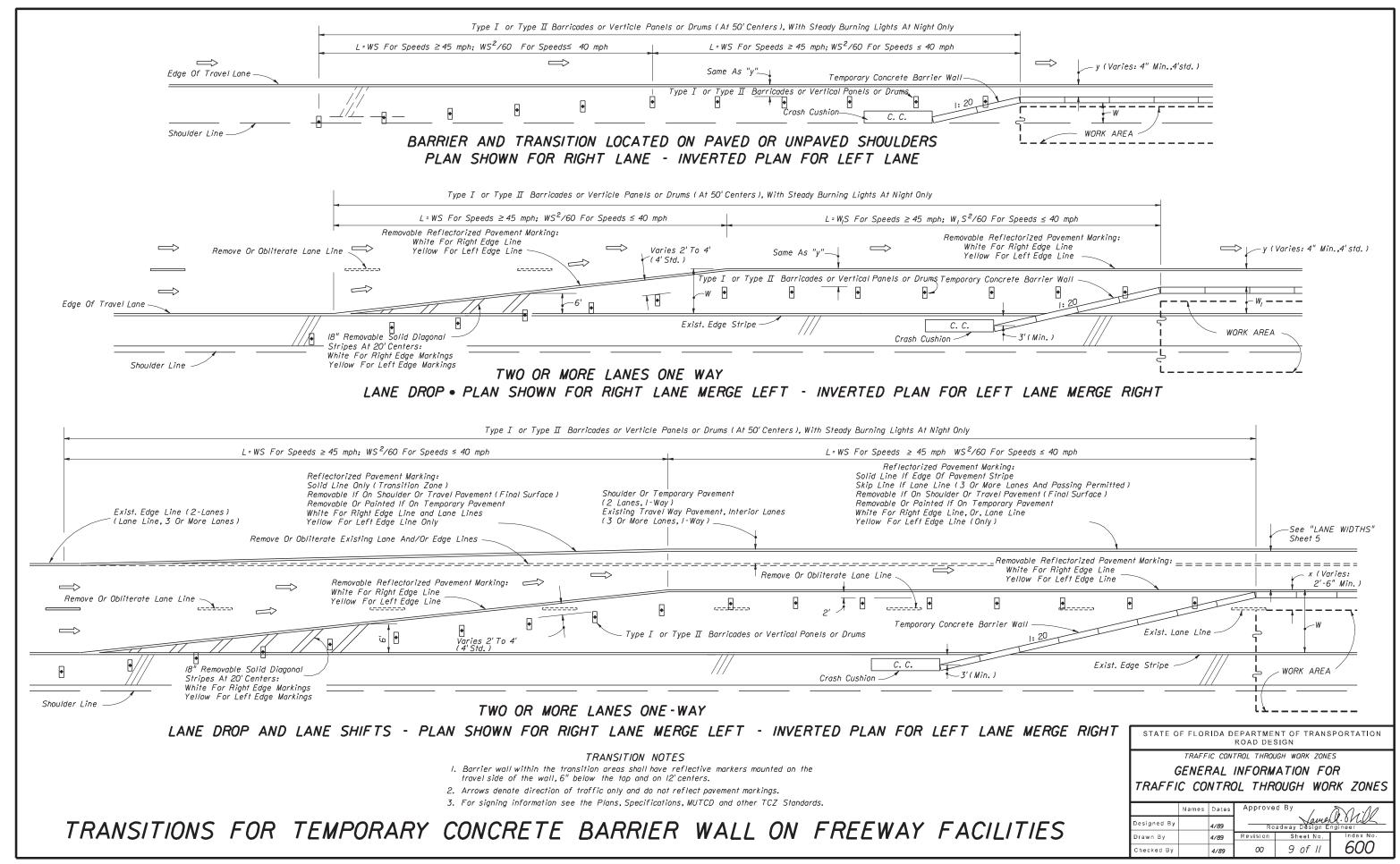
IDENTIFICATIONS - CHANNELIZING AND LIGHTING DEVICES AND ADVANCE WARNING ARROW PANEL MODES

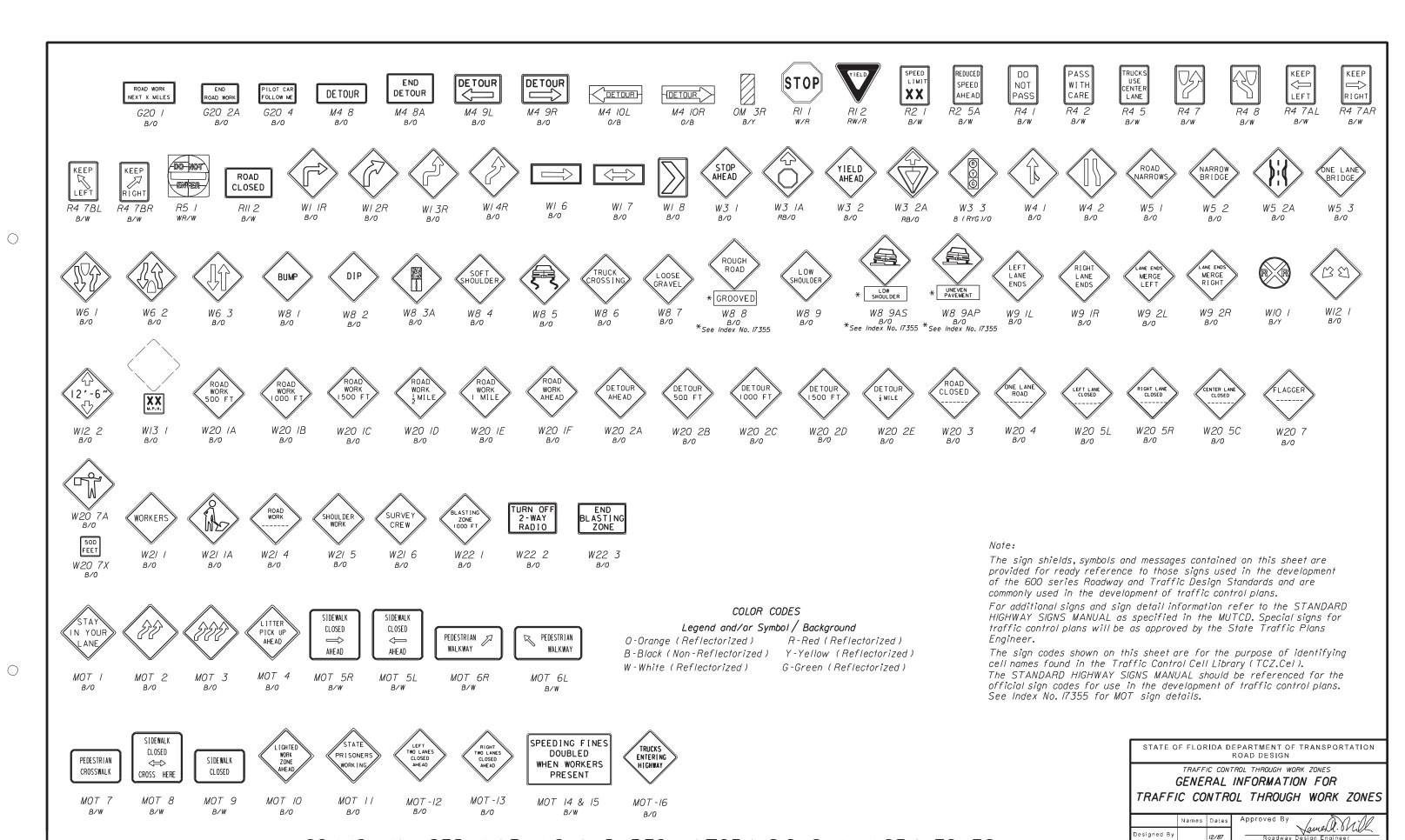
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES
GENERAL INFORMATION FOR

TRAFFIC CONTROL THROUGH WORK ZONES

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COMMONLY USED WARNING AND REGULATORY SIGNS IN WORK ZONES

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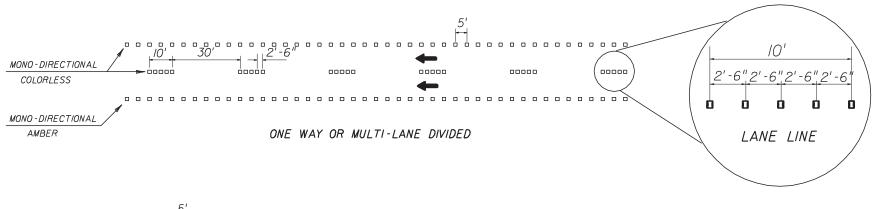
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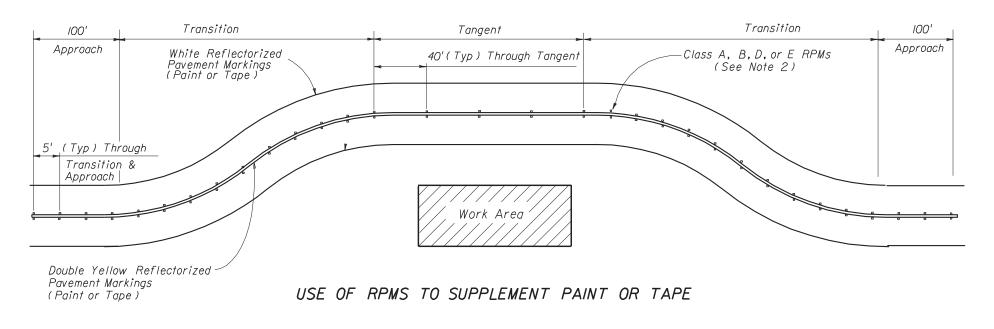




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2-LANE - 2 WAY

TYPICAL PLACEMENT OF REFLECTIVE PAVEMENT MARKERS IN LIEU OF TEMPORARY TAPE OR PAINT IN WORK ZONES



REFLECTIVE PAVEMENT MARKERS

RPM CLASS

APPLICATION

- Permanent Applications In Non-Traffic Areas Or Can Be Used In Work Zone Applications For Traffic And Non-Traffic Areas.
- Permanent Application In Traffic And Non-Traffic Areas Or Can Be Used In Work Zone Applications For Traffic And Non-Traffic Areas.
- D Work Zone Application Only, For Traffic And Non-Traffic Areas.
- Temporary Work Zone Application Only, Not Exceeding Five (5) Continuous Days, For Traffic And Non-Traffic Areas.

NOTES

- I. RPMs shall be installed as a supplement to all lane lines and the edge lines of of gore areas during construction. Placement of RPMs should be as shown in Index 17352 with the exception that Class D markers be placed at a maximum spacing of 5'center to center.
- 2. In work zones, CLASS A, B, or D RPMs may be used to form lane lines, edge lines and temporary gore areas, in lieu of tape or paint; however, tape or paint must be used in all transition areas in addition to RPMs. In short term work zones, where the RPMs will be used for five (5) days or less, CLASS "E" RPMs may be used to form lane or edge lines.
- 3. Basic color rule: colorless reflectors supplement white lines and amber reflectors supplement yellow lines.
- 4. To provide contrast on concrete pavement, or light asphalt, the five (5) colorless RPMs shall be followed by five black RPMs. The spacing between RPMs shall be 2'-6". Black RPMs will not be required for contrast with amber RPMs.
- 5. It shall be the contractors responsibility to replace damaged or missing RPMs.
- 6. RPMs used to supplement lane lines are to be paid for as Reflective Pavement Marker (Temporary), EA. RPM's used in lieu of temporary tape or paint are to be paid for as Removable Pavement Marking L.F.

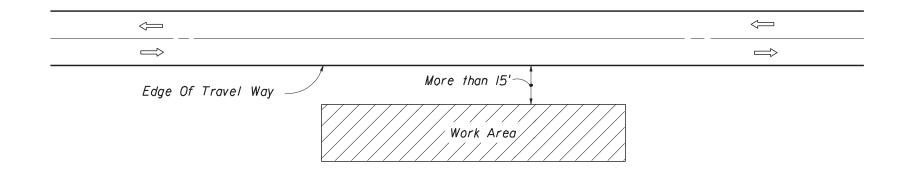
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

GENERAL INFORMATION FOR

TRAFFIC CONTROL THROUGH WORK ZONES

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- I. If the work operation requires that two or more work vehicles cross the 15' zone in any one hour, traffic control will be in conformance with Index No. 602.
- 2. No special signing is required.
- 3. Arrows denote direction of traffic only and do not reflect pavement markings.
- 4. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 5. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Landscaping Work
Utility Work
Fencing Work
Cleaning Drainage Structures
Reworking Ditches

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS AND THEIR ACTIVITIES ARE MORE THAN 15' FROM THE EDGE OF PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

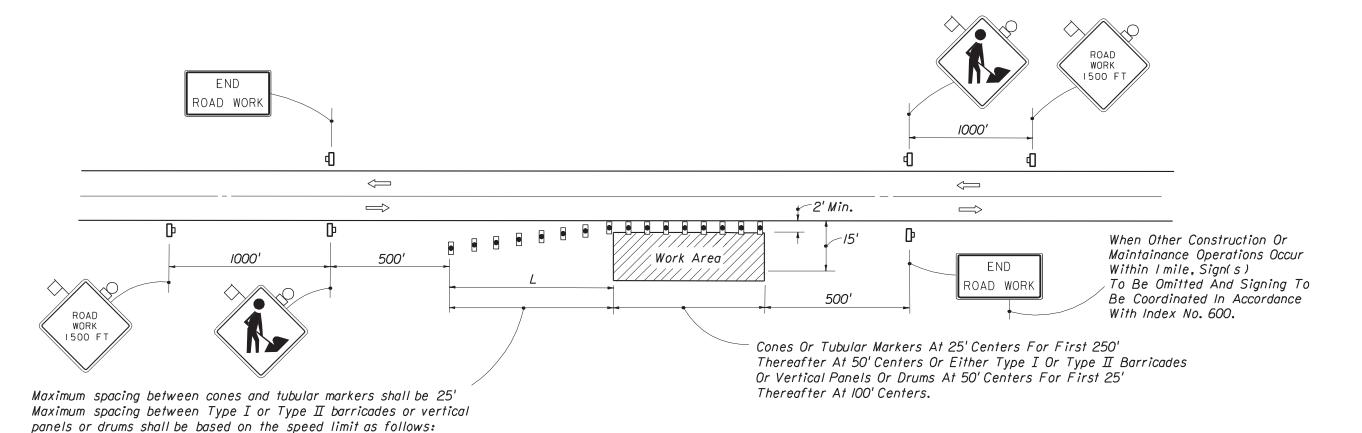
TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • RURAL

DAY OR NIGHT OPERATIONS

//// Work Area

SYMBOLS



I. All vehicles, equipment, workers (except flaggers) and their activities are restricted at all times to one side of the roadway.

2. If the work operation does not exceed 60 minutes, traffic control will be in conformance with Index No. 607.

- 3. When four or more work vehicles enter the through traffic lanes in a one hour period or less, the advanced FLAGGER sign shall be substitued for the WORKERS sign. For location of flaggers and FLAGGER signs, see Index No. 603
- 4. The first two warning signs shall have a 18" x 18"(min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.

- 5. The WORKERS legend sign may be substituted for the symbol sign.
- 6. All signs shall be post mounted if the work operation time exceeds 12 hours.
- 7. L (min)=WS for speeds ≥ 45 mph

for speeds ≤ 40 mph

W = Width of shoulder in feet. 8' minimum.

S = Posted speed limit (mph)

- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. WORKERS sign to be removed or fully covered when no work is being performed.
- II. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 12. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Utility Work Culvert Extensions Side Slope Work Guardrail Work Landscaping Work Cleaning Drainage Structures Reworking Ditches Sign Installation And Maintenance Shoulder Repair

CONDITIONS

WHERE ANY VEHICLE. EQUIPMENT. WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

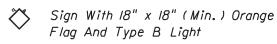
TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE. TWO-WAY • RURAL DAY OR NIGHT OPERATIONS

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SYMBOLS



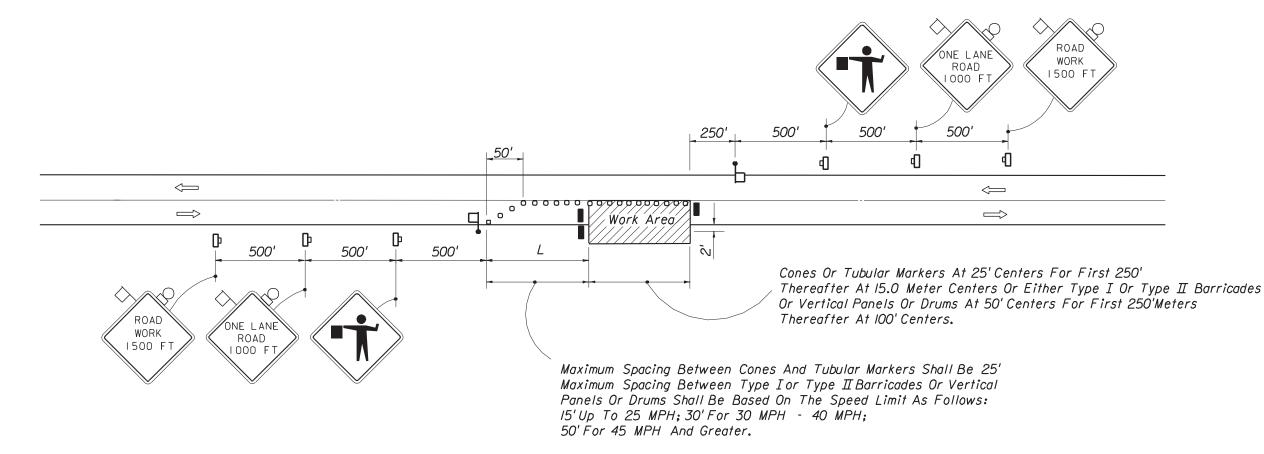


Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).

15' up to 25 MPH; 30' for 30 - 40 MPH;

50' for 45 MPH and greater.

Work Zone Sign



- I. Work operations shall be confined to one traffic lane, leaving the opposite lane open to traffic.
- 2. All vehicles, equipment, workers (except flaggers), and their activities are restricted at all times to one side of the roadway.
- 3. If the work operation does not exceed 60 minutes, traffic control will be in conformance with Index No. 607.
- Additional one-way control may be effected by the following means:
 (1) Flag-carrying vehicle; (2) Official vehicle;
 (3) Pilot vehicles; (4) Traffic signals.

When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.

5. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations. Type B Lights and Orange Flags are not required.

6. The FLAGGER legend sign may be substituted for the symbol sign.

7. $L (min) = \frac{WS}{2}$ for speeds ≥ 45 mph = $\frac{WS^2}{2}$ for speeds ≤ 40 mph

lhere.

W = Width of lateral transition in feet S = Posted speed limit (mph)

- 8. The ONE-LANE ROAD signs are to be fully covered and the FLAGGER signs either removed or fully covered when no work is being performed and the highway is open to two-way traffic.
- 9. Arrows denote direction of traffic only and do not reflect pavement markings.
- IO. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 12. For general TCZ requirements and additional information, refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Resurfacing Pavement Repair Utility Work Bridge Repair Guardrail Work

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • RURAL

OPERATIONS ONE DAYLIGHT

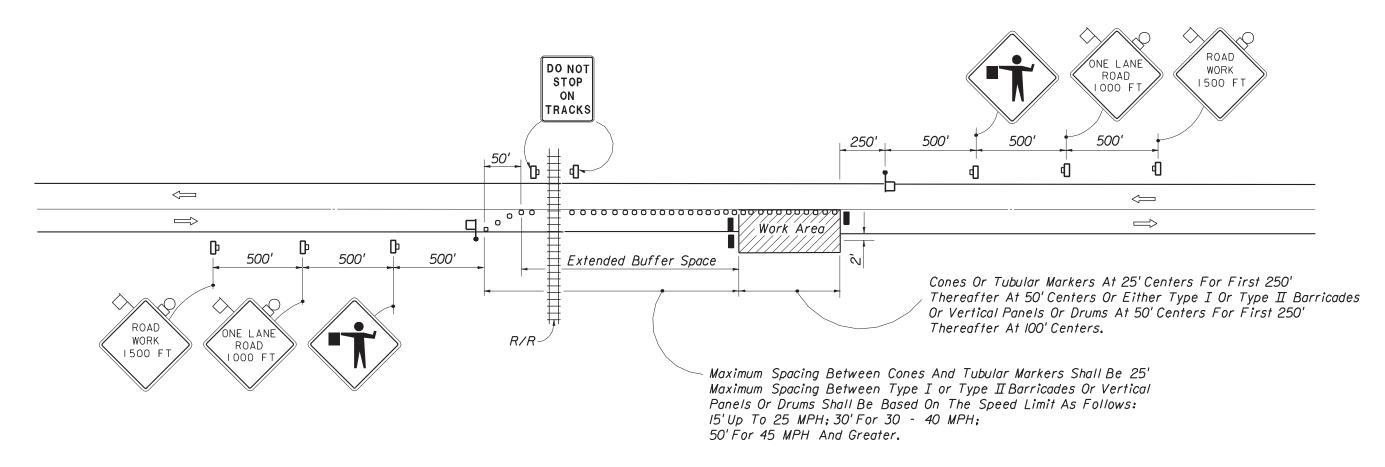
PERIOD OR LESS

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SYMBOLS

Work Area

- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum
- Type I Or Type II Barricade Or Vertical Panel
 Or Cone Or Tubular Marker Or Drum
- ₩ork Zone Sign
- □ Flagger



- I. Work operations shall be confined to one traffic lane, leaving the opposite lane open to traffic.
- 2. All vehicles, equipment, workers (except flaggers), and their activities are restricted at all times to one side of the roadway.
- 3. When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.
- 4. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations. Type B Lights and Orange Flags are not required.

5. The FLAGGER legend sign may be substituted for the symbol sign.

- 6. The ONE-LANE ROAD signs are to be fully covered and the FLAGGER signs either removed or fully covered when no work is being performed and the highway is open to two-way traffic.
- 7. Arrows denote direction of traffic only and do not reflect pavement markings.
- 8. Longitudinal dimensions are to be adjusted to fit field conditions, See Index No. 600.
- 9. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 10. For general TCZ requirements and additional information, refer to Index No. 600.
- II. Discontinuing of extended buffer space will not occur until the queue length plus 300' is reached.

TYPICAL APPLICATIONS

Pavement Resurfacing
Pavement Repair
Utility Work
Bridge Repair
Guardrail Work

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF PAVEMENT THAT REQUIRES A LANE CLOSURE IN THE VICINITY OF A RAILROAD CROSSING.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • RURAL

OPERATIONS ONE DAYLIGHT

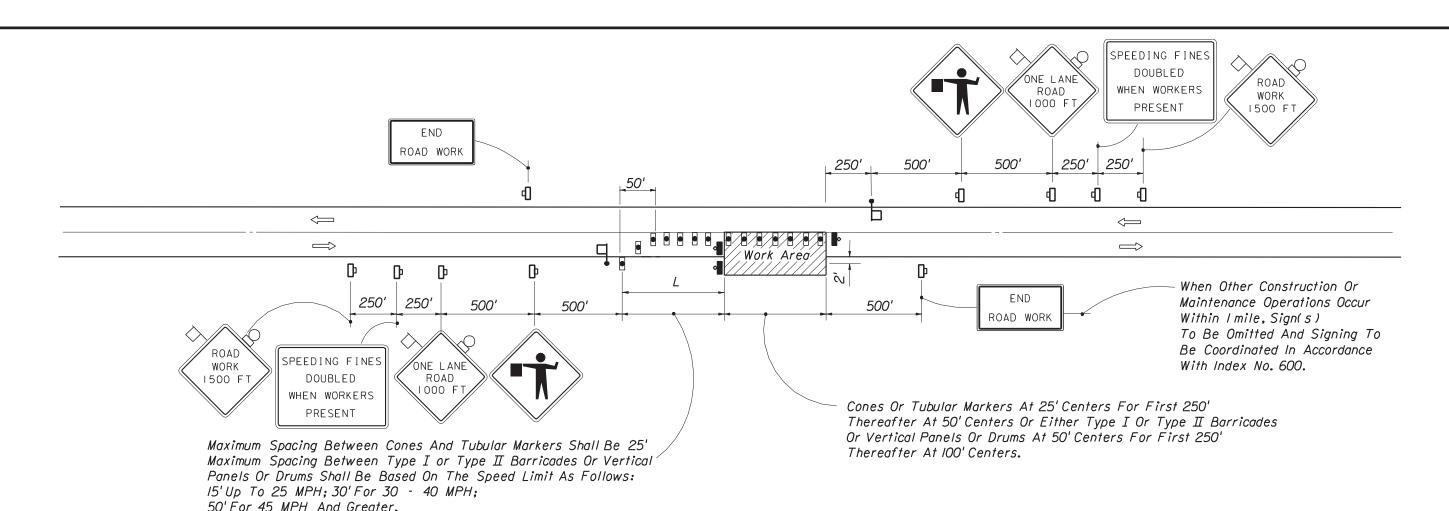
PERIOD OR LESS

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SYMBOLS

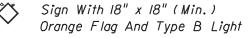
Work Area

- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I, Type II Or Type III Barricade
 Or Vertical Panel Or Drum
- Type I Or Type II Barricade Or Vertical Panel
 Or Cone Or Tubular Marker Or Drum
- ₩ork Zone Sign
- □ Flagger



SYMBOLS

Work Area



- Type I Or Type II Barricade Or Vertical Panel
 Or Drum (With Steady Burning Light At Night Only).
 (Tubular Markers May Be Used During Daylight Only.
 Cones May Be Used -See Index 600).
- Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum (With Flashing Light)
- ₩ork Zone Sign
- **□** Flagger

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- I. Construction operations shall be confined to one traffic lane, leaving the opposite lane open to traffic.
- 2. All vehicles, equipment, workers, (except flaggers) and their activities are restricted at all times to one side of the roadway.
- Additional one-way control may be effected by the following means:
 (1) Flag-carrying vehicle; (2) Official vehicle;
 (3) Pilot vehicles; (4) Traffic signals.

When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.

- 4. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.
- 5. The FLAGGER legend sign may be substituted for the symbol sign.
- 6. All signs shall be post mounted if the closure time exceeds 12 hours.

7. $L (min) = \frac{WS}{2}$ for speeds ≥ 45 mph = $\frac{WS^2}{120}$ for speeds ≤ 40 mph

Where:

W = Width of lateral transition in feet.

S = Posted speed limit (mph)

- 8. The ONE-LANE ROAD signs are to be fully covered and the FLAGGER signs either removed or fully covered when no work is being performed and the highway is open to two-way traffic.
- 9. Arrows denote direction of traffic only and do not reflect pavement markings.
- IO. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Repair Culvert Construction Utility Work Bridge Repair

Design Drawn

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF PAVEMENT

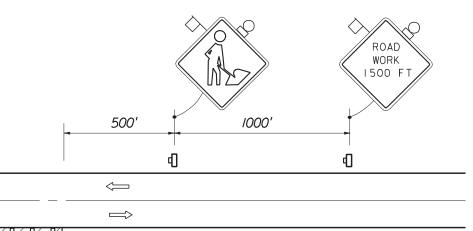
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

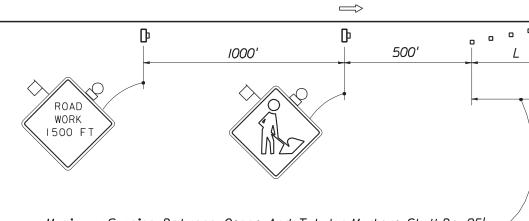
TWO-LANE, TWO-WAY RURAL
NIGHT OPERATIONS OR OPERATIONS
EXCEEDING ONE DAYLIGHT PERIOD

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Cones Or Tubular Markers At 25' Centers For First 250' Thereafter At 50 foot Centers Or Either Type I Or Type II Barricades Or Vertical Panels Or Drums At 50' Centers For First 250' Thereafter At 100' Centers.

Maximum Spacing Between Cones And Tubular Markers Shall Be 25' Maximum Spacing Between Type I Or Type II Barricades Or Vertical Panels Or Drums Shall Be Based On The Speed Limit As Follows: 15' Up To 25 MPH; 30' For 30 MPH - 40 MPH; 50' For 45 MPH And Greater.

GENERAL NOTES

- I. All vehicles, equipment, workers (except flaggers), and their activities are restricted at all times to one side of the roadway.
- 2. If the work operation does not exceed 60 minutes, traffic control will be in conformance with Index No. 607.
- 3. If the work operation encroaches on the through traffic lanes or when four or more work vehicles enter the through traffic lanes in a one hour period flaggers shall be provided and the advanced FLAGGER sign shall be substituted for the WORKERS sign. For location of flaggers and FLAGGER signs see Index No. 603.
- 4. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating Mesh signs may be used for (Daylight Only) operations. Type B Lights and Orange Flags are not required.
- 5. The WORKERS legend sign may be substituted for the symbol sign.
- \star 6. Where work activities within 2' of the edge of pavement are incidental (ie. Mowing, Litter Removal) the engineer may delete requirements for cones and signs provided a vehicle with flashing warning lights is present.

7. $L (min.) = \underline{WS} \text{ for speeds } \ge 45 \text{ mph}$ WS² for speeds ≤ 40 mph

Where:

Work Area

W = Width of shoulder in feet. 8' minimum. S = Posted speed limit (mph)

- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- II. For general TCZ requirements and additional information, refer to Index No. 600.

TYPICAL APPLICATIONS

Shoulder And Slope Work Utility Work Guardrail Work Landscape Work Delineator Installation And Maintenance

- * Mowing
- * Litter Removal

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES REQUIRE AN INTERMITTENT OR CONTINUOUS MOVING OPERATION ON THE SHOULDER OR SHOULDER AND SLOPES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE. TWO-WAY . RURAL MOVING OPERATIONS - DAYLIGHT ONLY

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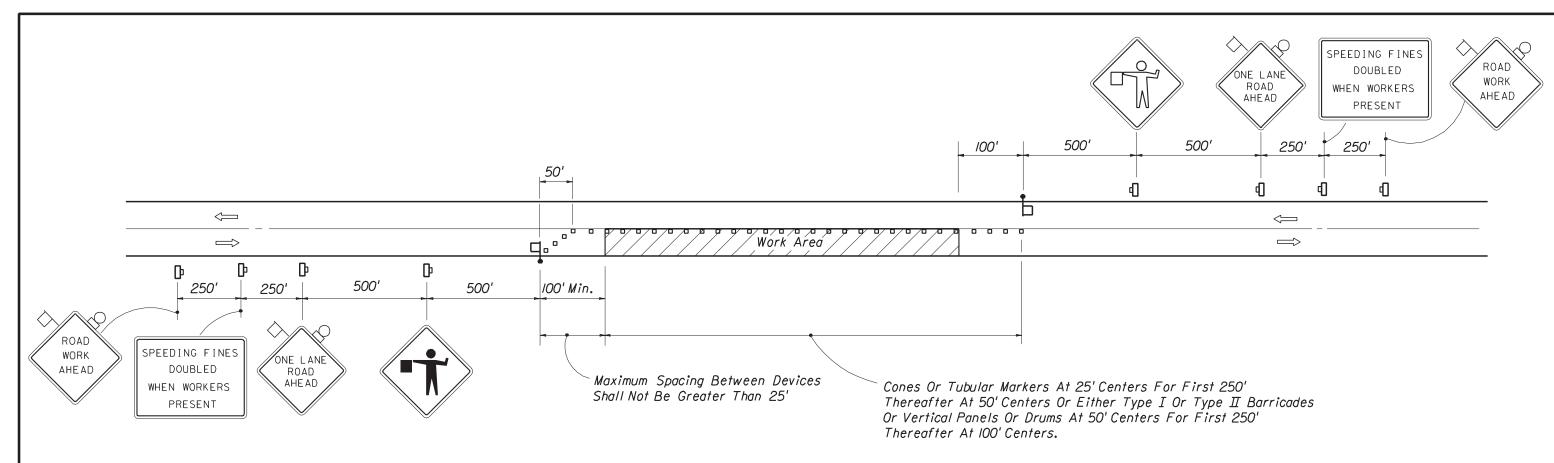
Work Area

Sign With 18" x 18" (Min.) Orange Flag And Type B Light

Type I Or Type II Barricade Or Vertical Panel Or Cone Or Tubular Marker Or Drum

Work Zone Sign

SYMBOLS



- All vehicles, equipment, workers (except flaggers), and their activities are restricted at all times to one side of the roadway.
- 2. Minimum length of work area is 200'. Maximum length to be determined by the Engineer, but in no case to exceed the length of one-half $(\frac{1}{2})$ days operation or 2 miles whichever is less.
- 3. If the work operation does not exceed 60 minutes, traffic control will be in conformance with Index No. 607.
- 4. Additional one-way control may be effected by the following means:
 (1) Flag-carrying vehicle; (2) Official vehicle;
 (3) Pilot vehicles; (4) Traffic signals.

When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.

- SYMBOLS
- Work Area
- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel
 Or Cone Or Tubular Marker Or Drum
- ₩ork Zone Sign
- **□** Flagger

- 5. The first two warning signs shall have an I8" x I8" orange flag and a Type B light attached and operating at all times.

 Mesh signs may be used for (Daylight Only) operations.

 Type B Lights and Orange Flags are not required.
- 6. The FLAGGER legend sign may be substituted for the symbol sign.
- 7. The ONE LANE ROAD AHEAD and FLAGGER signs are to be removed or fully covered when no work is being performed and the highway is open to two-way traffic.
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- IO. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- II. For general TCZ requirements and additional information, refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Repair
Pavement Surfacing
Utility Work
Delineator Maintenance
Crack Sealing
Core Boring

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES REQUIRE AN INTERMITTENT OR CONTINUOUS MOVING OPERATION ON THE PAVEMENT WHERE THE AVERAGE SPEED OF MOVEMENT IS LESS THAN 4 MILES PER HOUR

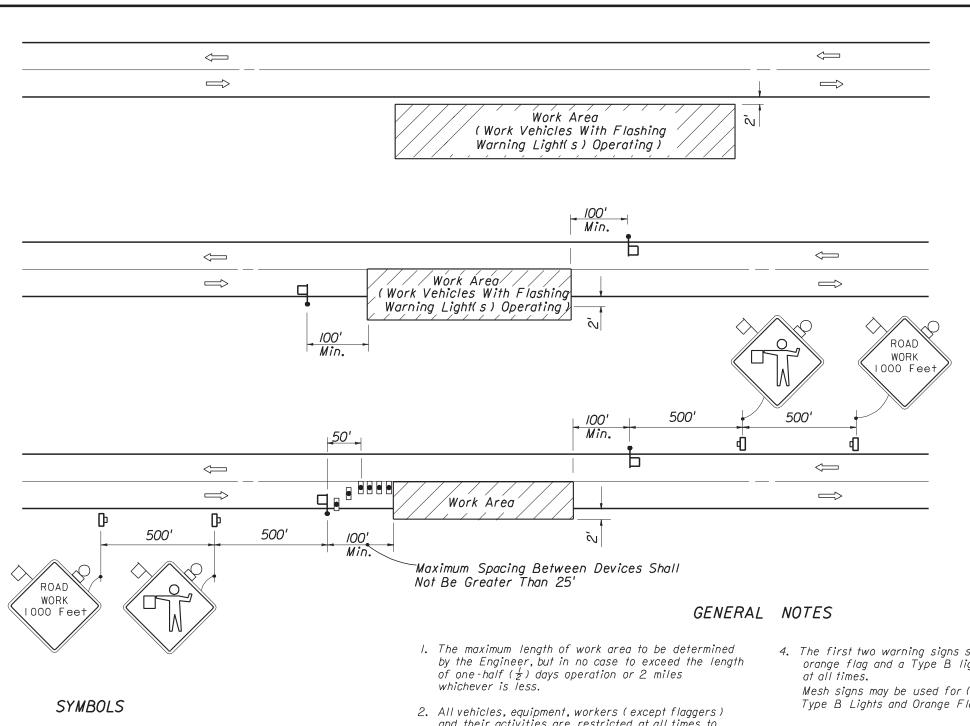
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • RURAL

MOVING OPERATIONS - DAYLIGHT ONLY

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CONDITIONS

FOR ANY OPERATION THAT IS 2' OR MORE OUTSIDE THE EDGE OF THE PAVEMENT FOR A PERIOD OF LESS THAN 60 MINUTES.

CONDITIONS

FOR ANY OPERATION THAT ENCROACHES IN THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF THE PAVEMENT FOR A PERIOD OF 15 MINUTES OR LESS

CONDITIONS

FOR ANY OPERATION THAT ENCROACHES IN THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF THE PAVEMENT FOR A PERIOD IN EXCESS OF 15 MINUTES BUT LESS THAN 60 MINUTES.

TYPICAL APPLICATIONS

Marking Patches Field Patches String Line Utility Work Cleaning Up Debris On Pavement Pavement Coring And Straight Edging

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

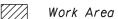
TRAFFIC CONTROL THROUGH WORK ZONES
TWO-LANE TWO-WAY • RURAL SHORTTIME DAY OR NIGHT OPERATIONS

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- and their activities are restricted at all times to one side of the roadway.
- 3. Additional one-way control may be effected by the following means: (1) Flag-carrying vehicle; (2) Official vehicle; (3) Pilot vehicles: (4) Traffic signals.

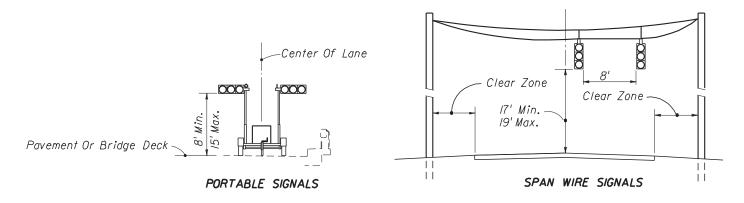
When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.

- 4. The first two warning signs shall have an I8" x I8"(min.) orange flag and a Type B light attached and operating
 - Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.
- 5. The FLAGGER legend sign may be substituted for the symbol sign.
- 6. Arrows denote direction of traffic only and do not reflect pavement markings.
- 7. Longitudinal dimensions are to be adjusted to fit field conditions, See Index No. 600.
- 8. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 9. For general TCZ requirements and additional information refer to Index No. 600.



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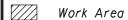
- Sign With 18" x 18" (Min.) Orange Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- Work Zone Sign
- Flagger



SIGNAL MOUNT DETAILS

GENERAL NOTES

SYMBOLS



- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- ₩ork Zone Sign
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only).
- N Type Ⅲ Barricade
- Stop Bar
- **□** Flagger
- Portable Signal

- I. Work operations shall be confined to one traffic lane, except for haul road crossings, leaving the opposite lane open to traffic.
- 2. All vehicles, equipment, workers (except flaggers) and their activities are restricted at all times to one side of the roadway, except for haul road crossings.
- 3. The installation and timing of signals shall be approved by the District Traffic Operations Engineer prior to signals being placed in operation.

Where sight distance to the signal is limited, the signals may be mounted on span wire at the discretion of the Engineer.

The maximum distance between portable traffic signals (receiver/controllers) shall be .25 mile, however, in no case shall the distance exceed the maximum distance at which the remote operator (transmitter) can positively and and safely operate both portable signals.

- 4. Flaggers to supplement the signal operator/flagger shall be used when needed to assure safe movements between traffic and operating equipment, as determined by the Engineer.
- 5. The first two warning signs shall have a 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.
- 6. When needed, an additional warning sign may be installed in advance of the ROAD WORK AHEAD sign. The distance between successive signs shall be 500'
- 7. The SIGNAL AHEAD legend sign may be substituted for the symbol sign.

- 8. All signs shall be post mounted if the closure time exceeds 12 hours.
- 9. SIGNAL AHEAD and EQUIPMENT CROSSING AHEAD signs are to be removed or fully covered when no work is being performed and the highway is open to two-way traffic. Type III Barricades shall be in place to block haul road access when the haul road is not in operation and a flagger/signal operator is not on duty, except when the haul road is an existing properly marked road.
- IO. Arrows denote direction of traffic only and do not reflect pavement markings.
- II. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- I2. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 13. For general TCZ requirements and additional information refer to Index No. 600.
- 14. Span wire signals are to be used only in work zones with workers present, where the contractor can monitor signal operation and maintain traffic with flaggers in the event of a power failure.

TYPICAL APPLICATION

Pavement Repair
Shoulder & Roadside Work
Bridge Work
Box Culvert Work
Drainage Work
Utility Work
Haul Road Crossing

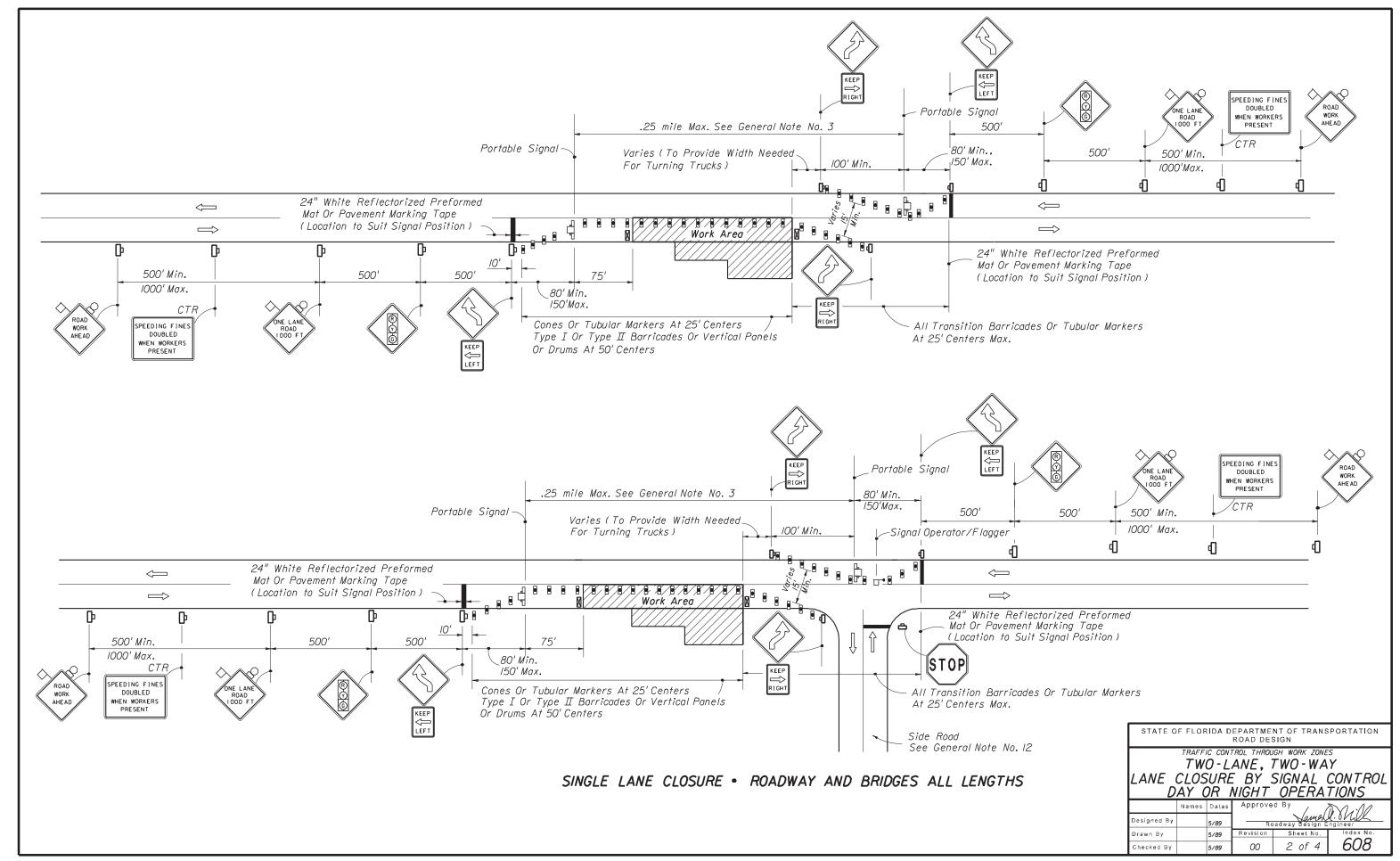
CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES WILL ENCROACH ON ONE LANE OR MOMENTARILY ENCROACH ON BOTH LANES OF A TWO-LANE TWO-WAY ROADWAY AND TRAFFIC SIGNALS ARE NEEDED.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

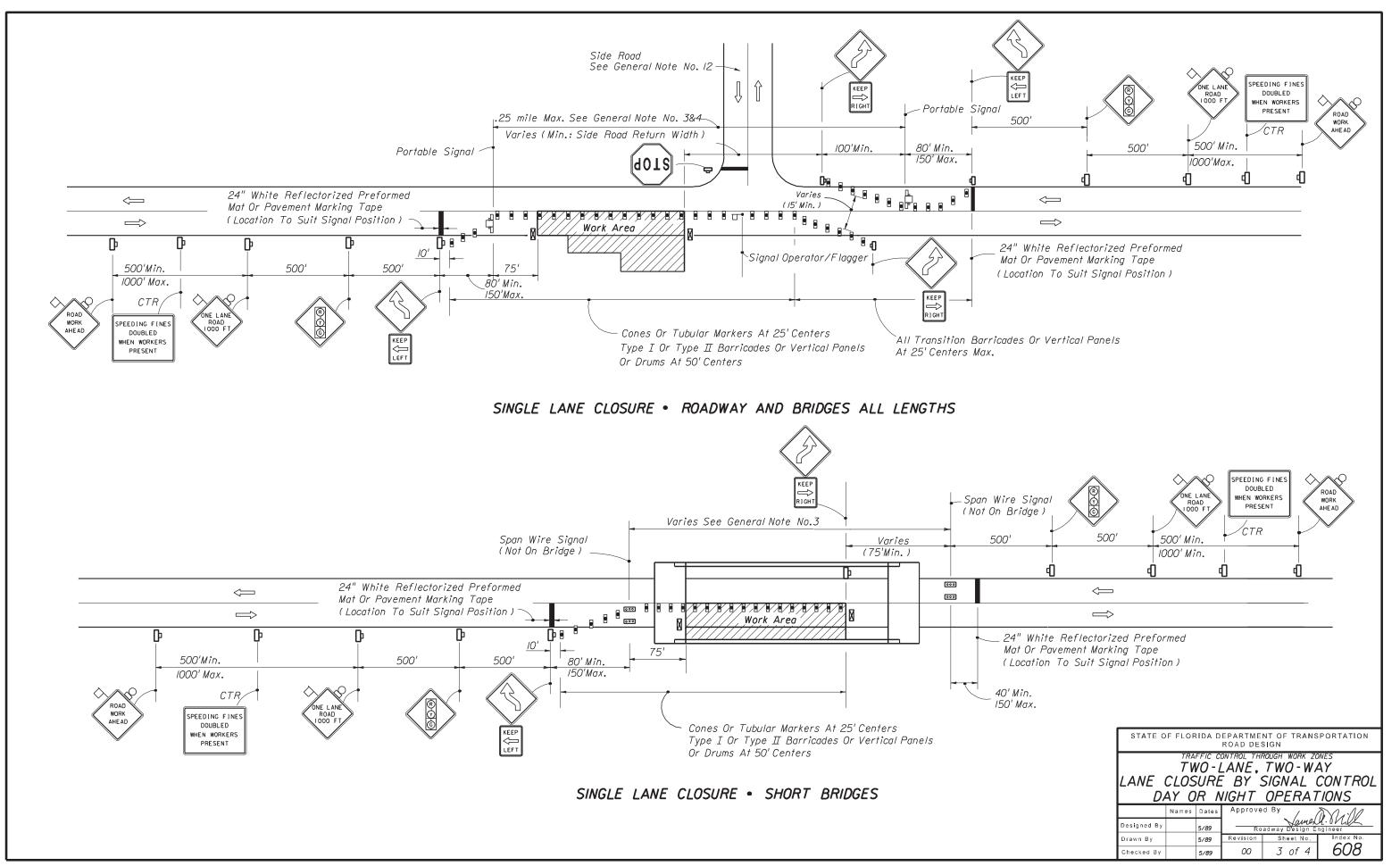
TRAFFIC CONTROL THROUGH WORK ZONES
TWO-LANE, TWO-WAY
LANE CLOSURE BY SIGNAL CONTROL
DAY OR NIGHT OPERATIONS

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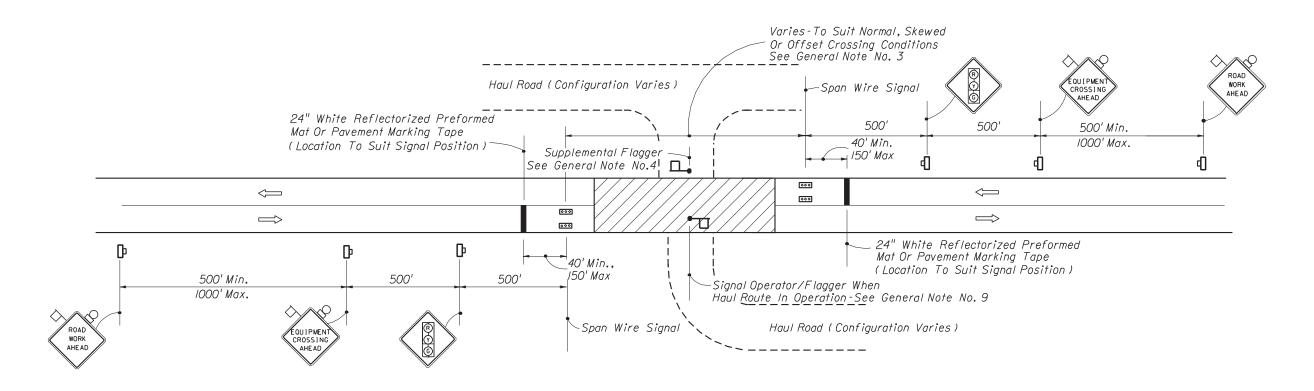


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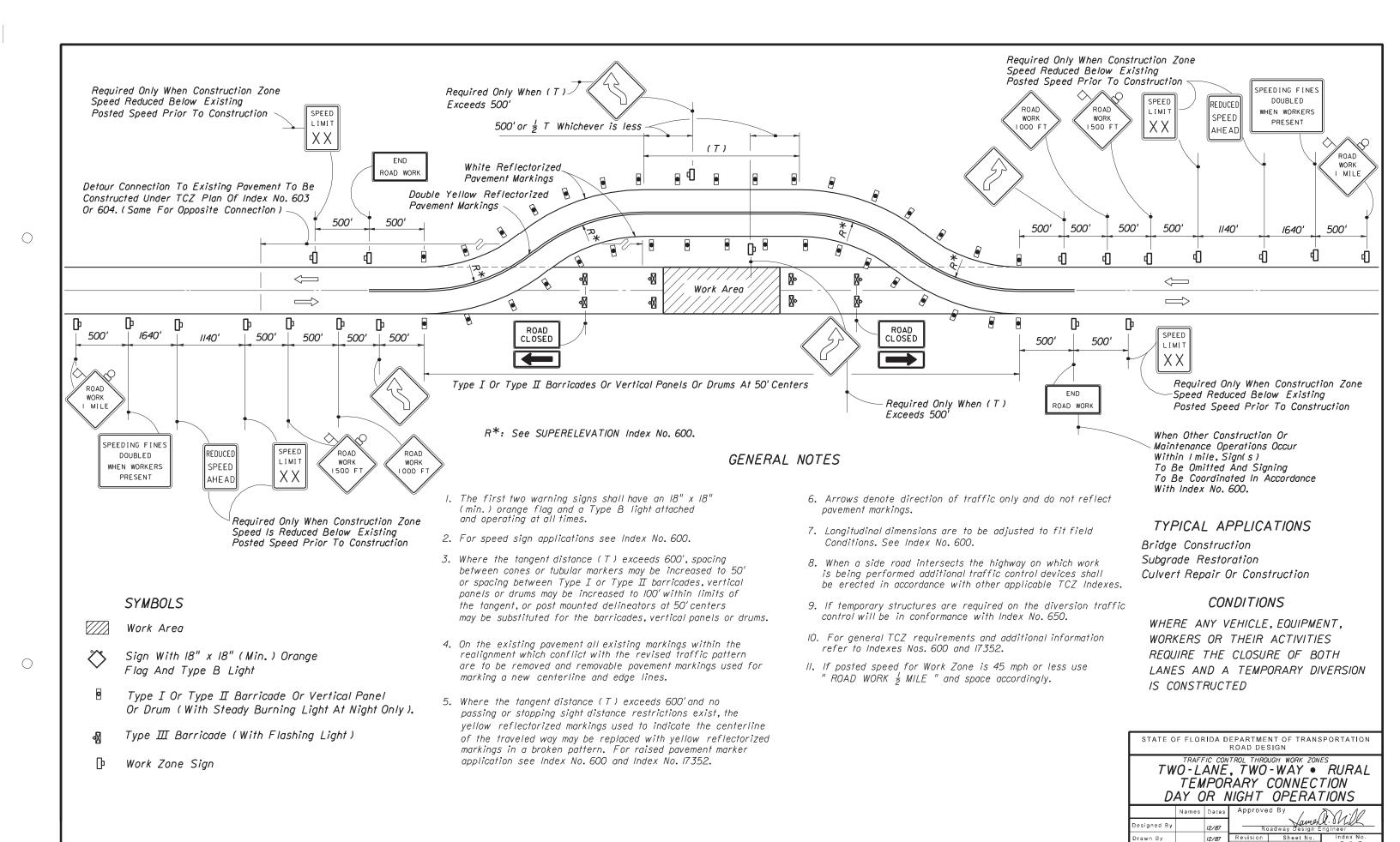
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MOMENTARY ROADWAY CLOSURE . HAUL ROUTE CROSSING

TRAFFIC CONTROL THROUGH WORK ZONES
TWO-LANE, TWO-WAY
LANE CLOSURE BY SIGNAL CONTROL
DAY OR NIGHT OPERATIONS

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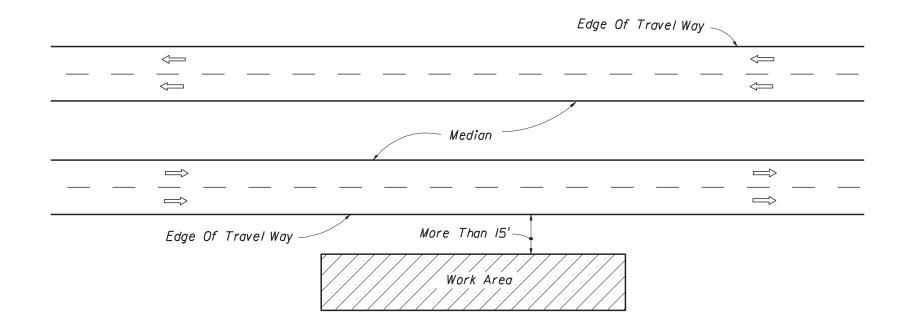


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609



- I. If the work operation requires that two or more work vehicles cross the 15' zone in any one hour, traffic control will be in conformance with Index No. 602 undivided or Index No. 611 divided.
- 2. No special signing is required.
- 3. This index also applies when work is being performed on a multilane undivided highway.
- 4. This index also applies to work performed in the median more than 15' from edge of travel way, both roadways.
- 5. Arrows denote direction of traffic only and do not reflect pavement markings.
- 6. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 7. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Landscaping Work
Utility Work
Fencing Work
Cleaning Drainage Structures
Reworking Ditches

CONDITIONS

WHERE ALL VEHICLES, EQUIPMENT, WORKERS AND THEIR ACTIVITIES ARE MORE THAN 15' FROM THE EDGE OF PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

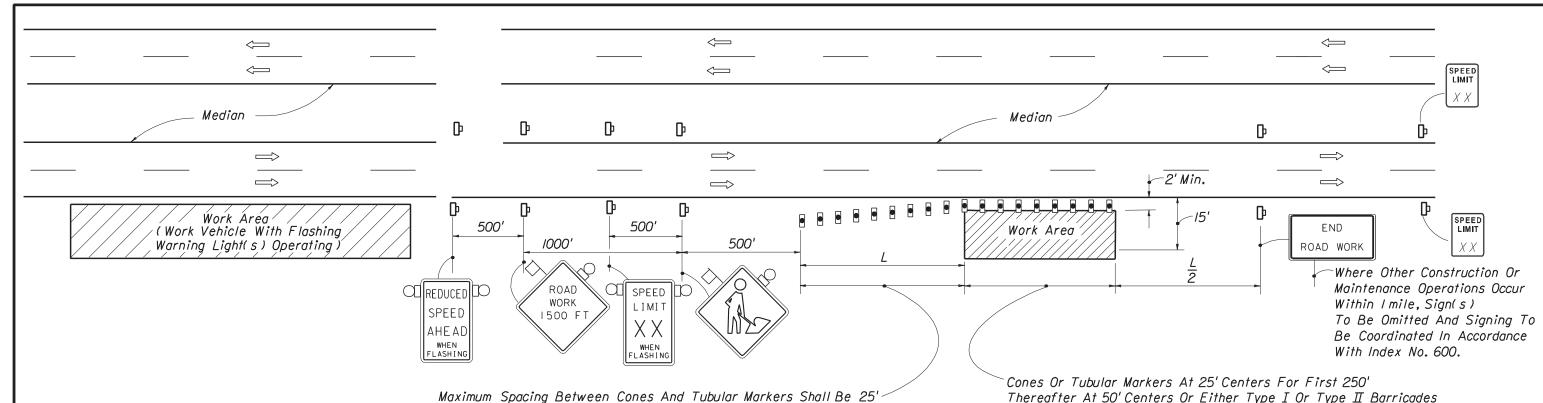
MULTILANE DIVIDED OR UNDIVIDED

RURAL • DAY OR NIGHT OPERATIONS

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SYMBOLS

//// Work Area



CONDITIONS

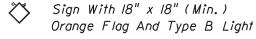
WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF PAVEMENT FOR A PERIOD OF LESS THAN 60 MINUTES

SYMBOLS

Work Area

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- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- ₩ork Zone Sign

Maximum Spacing Between Type I Or Type II Barricades Or Vertical Panels Or Drums Shall Be Based On The Speed Limit As Follows: 15'Up To 25 MPH; 30' For 30 - 40 MPH; 50' For 45 MPH And Greater.

GENERAL NOTES

- I. All vehicles, equipment, workers and their activities are restricted at all times to one side of the roadway.
- 2. If the work operation encroaches on the through traffic lanes or when four or more work vehicles enter the through traffic lanes in a one hour period a flagger shall be provided and a FLAGGER sign shall be substituted for the WORKERS sign. The flagger shall be positioned at the point of vehicle entry or departure from the work area.
- 3. This TCZ plan also applies to work performed in the median more than 2' but less than 15' from the edge of either pavement.
- 4. The first two warning signs, each side, shall have a 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.

- 5. The WORKERS legend sign may be substituted for the symbol sign.
- 6. $L (min) = \frac{WS}{2}$ for speeds ≥ 45 mph = $\frac{WS}{2}$ for speeds ≤ 40 mph

Where

W = Width of lateral transition in feet.

S = Posted speed limit

7. Arrows denote direction of traffic only and do not reflect pavement markings.

Thereafter At 100' Centers.

Or Vertical Panels Or Drums At 50' Centers For First 250'

- 8. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 9. When work is being performed on a multilane undivided roadway the signs normally mounted in the median (as shown) shall be omitted.
- 10. WORKERS signs to be removed or fully covered when no work is being performed.
- II. END ROAD WORK signs required only when work exceeds one daylight period.
- 12. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 13. If the work operation does not exceed 15 minutes, signs, barricades, vertical panels, cones, tubular markers, or drums will not be required provided vehicles in the work area have warning light(s) operating.
- For general TCZ requirements and additional information refer to Index No. 600.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF PAVEMENT FOR A PERIOD OF 60 MINUTES OR GREATER

TYPICAL APPLICATIONS

Utility Work
Culvert Extensions
Side Slope Work
Guardrail Work
Landscaping Work
Cleaning Drainage Structures
Reworking Ditches
Sign Installation And Maintenance
Shoulder Repair

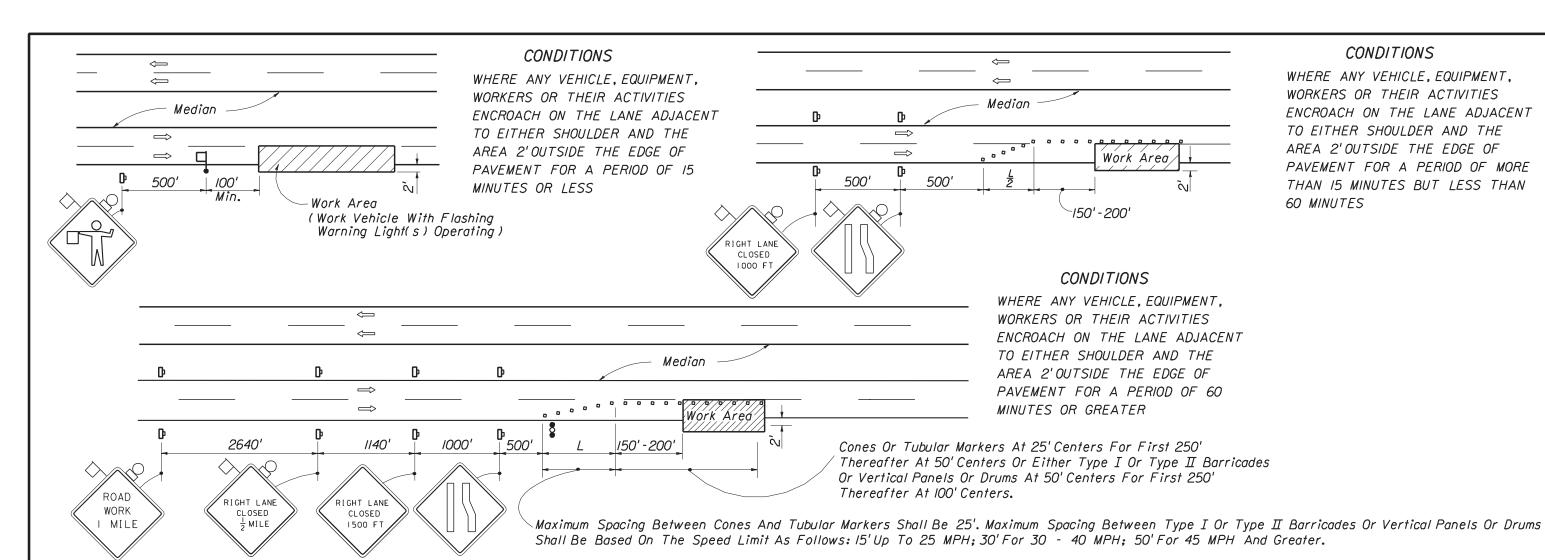
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

MULTILANE DIVIDED OR UNDIVIDED

RURAL • DAY OR NIGHT OPERATIONS

	Names	Dates	Approve		Mill
Designed By		12/87	Ro	adway Design E	
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- I. Work operations shall be confined to one traffic lane, leaving the adjacent lane open to traffic.
- 2. All vehicles, equipment, workers, and their activities are restricted at all times to one side of the roadway.
- 3. The first two warning signs, each side, shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations. Type B Lights and Orange Flags are not required.

- 4. On undivided highways the median signs as shown are to be omitted.
- 5. When work is performed in the median lane on divided highways the barricading plan is inverted and left lane closed and lane reduction signs substituted for the right lane closed and lane reduction signs.

The same applies to undivided highways with the following exceptions:
(a) Work shall be confined within one median lane. (b) Additional barricades, cones, or drums shall be placed along the centerline abutting the work area and across the trailing end of the work area.

When work on undivided highways occurs across the centerline so as to encroach on both median lanes, the inverted plan is applied to the approach of both roadways.

6. The RIGHT (LEFT) LANE CLOSED signs are to be removed or fully covered when no work is being performed and the highway is open to traffic.

- 7. L (min.)=Length of taper feet: = WS for speeds ≥ 45 mph
 - $=\frac{WS^2}{60}$ for speeds ≤ 40 mph

Where:

- W = Width of lateral transition in feet
- S = Posted speed limit
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- IO. When work is being performed on a multilane undivided roadway the signs normally mounted in the median (as shown) shall be omitted.
- II. This TCZ plan does not apply when work is being performed in the middle or inside lane(s) of a six or more lane highway. See Indexes Nos. 616 and 617.
- I2. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- For general TCZ requirements and additional information, refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Resurfacing
Pavement Repair
Utility Work
Bridge Repair
Guardrail Work
Pavement Coring And Straight Edging

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

MULTILANE, DIVIDED AND UNDIVIDED

RURAL • OPERATIONS ONE

DAYLIGHT PERIOD OR LESS

	Names	Dates	Approve	. /	Marile
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Advance Warning Arrow Panel

Work Zone Sign

Flagger

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Sign With 18" x 18" (Min.)

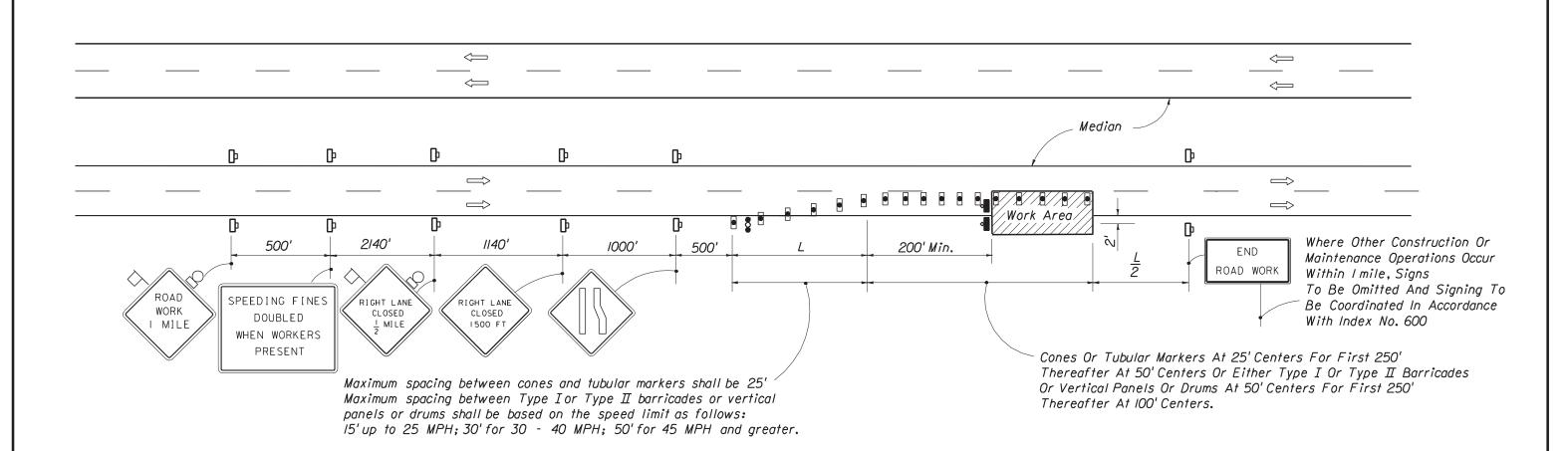
Orange Flag And Type B Light

Or Cone Or Tubular Marker Or Drum

Type I Or Type II Barricade Or Vertical Panel

SYMBOLS

Work Area



- Work operations shall be confined to one traffic lane, leaving the adjacent lane open to traffic.
- 2. All vehicles, equipment, workers and their activities are restricted at all times to one side of the roadway.
- 3. The first two warning signs, each side, shall have a 18" x 18"(min.) orange flag and a Type B light attached and operating at all times.
- 4. All signs shall be post mounted if the closure time exceeds 12 hours.
- 5. On undivided highways the median signs as shown are to be omitted.
- 6. When work is performed in the median lane on divided highways the barricading plan is inverted and left lane closed and lane reduction signs substituted for the right lane closed and lane reduction signs.

The same applies to undivided highways with the following exceptions:
(a) Work shall be confined within one median lane. (b) Additional barricades, cones, or drums shall be placed along the centerline abutting the work area and across the trailing end of the work area.

When work on undivided highways occurs across the centerline so as to encroach on both median lanes, the inverted plan is applied to the approach of both roadways.

7. Signs and traffic control devices are to be modified in accordance with INTERMITTENT WORK STOPPAGE details (sheet 2 of 2) when no work is being performed and the highway is open to traffic.

8. L (min.) = Length of taper in feet:

= WS for speeds ≥ 45 mph

= $\frac{WS}{60}^2$ for speeds ≤ 40 mph

Where:

W = Width of lateral transition in feet S = Posted speed limit (mph).

- 9. Arrows denote direction of traffic only and do not reflect pavement markings.
- 10. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. When work is being performed on a multilane undivided roadway the signs normally mounted in the median (as shown) shall be omitted.
- 12. When a side road intersects the highway on which work is being performed, additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- I3. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Resurfacing Pavement Repair Utility Work Bridge Repair Guardrail Work

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT,
WORKERS OR THEIR ACTIVITIES
ENCROACH ON THE LANE ADJACENT
TO EITHER SHOULDER AND THE
AREA 2'OUTSIDE THE EDGE OF
PAVEMENT

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

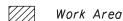
MULTILANE, DIVIDED AND UNDIVIDED • RURAL

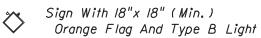
NIGHT OPERATIONS OR OPERATIONS

EXCEEDING ONE DAYLIGHT PERIOD

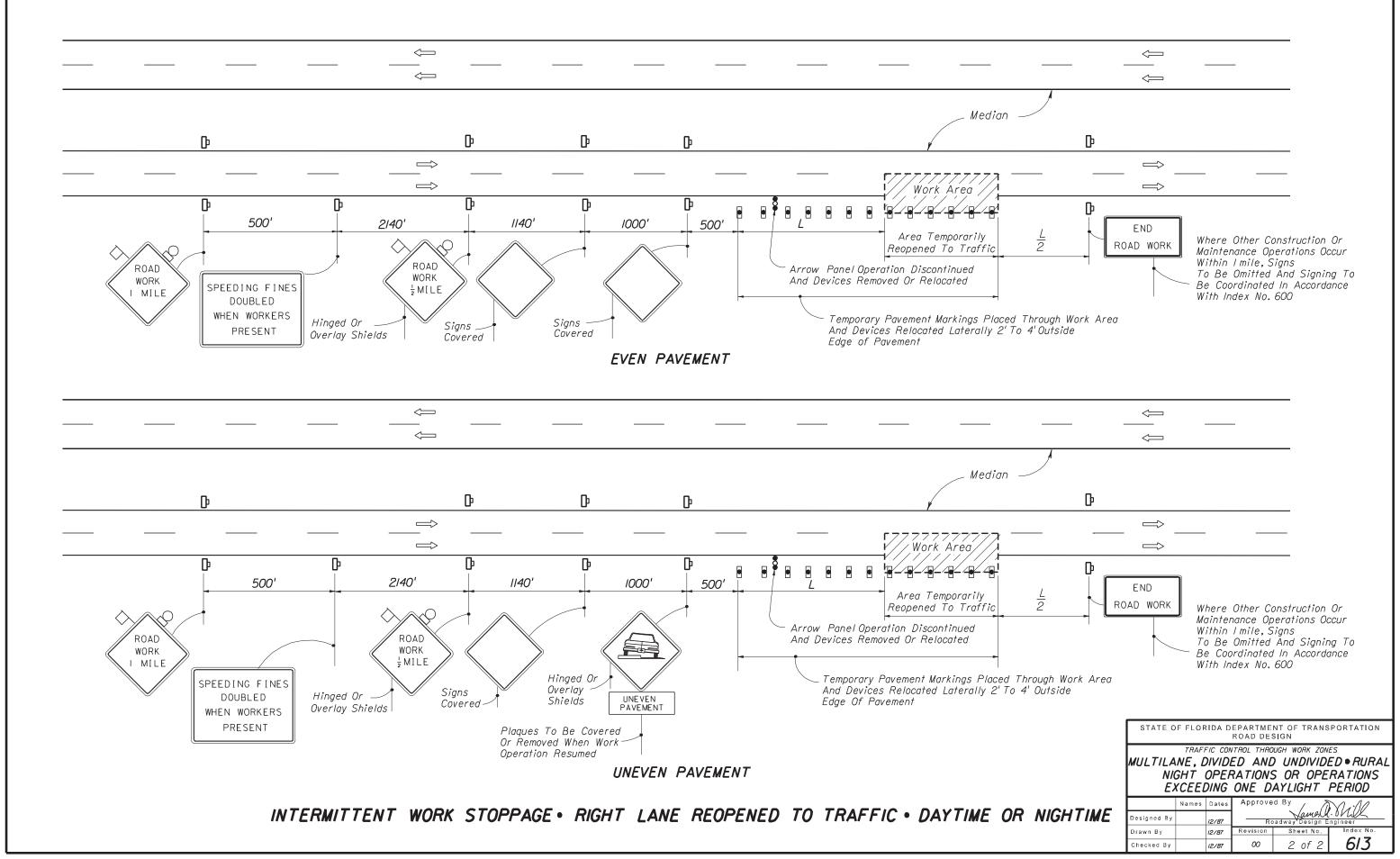
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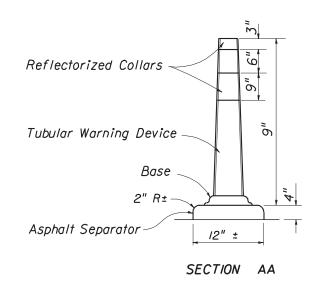
SYMBOLS

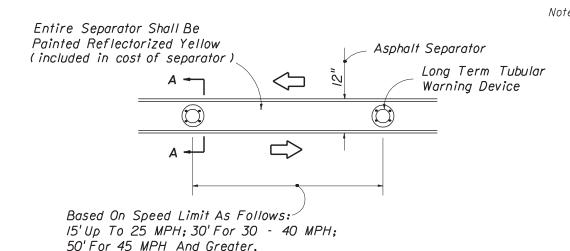




- Type I Or Type II Barricade Or Vertical Panel
 Or Drum (With Steady Burning Light At Night Only).
 (Tubular Markers May Be Used During Daylight Only.
 Cones May Be Used -See Index 600).
- Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum (With Flashing Light)
- ₩ork Zone Sign
- Advance Warning Arrow Panel







PLAN

Notes: (a) The tubular device is to be made of a flexible material or have a flexible joint at the base such that it will not cause damage to vehicles upon impact and will return to its original shape after being struck by a 5000 lb. vehicle at a velocity of 75'/sec.

- (b) The tubular device shall be orange with two white reflectorized collars.
- (c) The tubular device may be attached by bituminous adhesive or other methods approved by the Engineer.
- (d) Reflectorized materials shall have a smooth sealed outer surface which will display the same approximate color day and night.
- (e) 12" openings for drainage will be constructed in the separator island every 25' in areas with grades of 1% or less or every 50' in areas with arades over 1% as directed by the Engineer.

DETAIL OF TEMPORARY ASPHALT TRAFFIC SEPARATOR

APPLICATIONS

Scheme 1: Restricted Construction Limits

Scheme 2: Unrestricted Construction Limits And Light To Moderate Traffic

Scheme 3: Unrestricted Construction Limits

And Moderate To Heavy Traffic

Construction Limits Are The Outward Beginning Or Ending

Of Lane Reductions

Unless A Specific Scheme Is Called For In The Plans, Scheme Selection Shall Be At The Contractors Option And As Approved By The

Engineer

Where:

Where:

CONDITIONS

WHERE ANY VEHICLE. EQUIPMENT. WORKERS OR THEIR ACTIVITIES REQUIRE THE CLOSURE OF ONE ROADWAY AND THE OPPOSING ROADWAY IS CONVERTED TO TEMPORARY TWO-WAY TRAVEL BY WAY OF CROSSOVERS

GENERAL NOTES

SYMBOLS

Work Area

 \bigcirc

Sign With 18" x 18" (Min.) Orange Flag And Type B Light

- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only, Cones May Be Used -See Index 600).
- Work Zone Sign
- Advance Warning Arrow Panel

- I. All vehicles, equipment, workers and their activities are restricted at all times to one side of the highway.
- 2. The first two warning signs, each side, shall have a 18" x 18" orange flag and a Type B light attached and operating at all times.
- 3. All signs shall be post mounted.
- 4. TWO-WAY TRAFFIC sign(s) shall be repeated every $\frac{1}{4}$ mile in each direction, throughout the tangent distance (T).
- 5. L (min.)=WS for speeds ≥ 45 mph

= WS² for speeds ≤ 40 mph

Where:

W = Width of lateral transition in feet. S = Posted speed limit (mph).

6. Where the tangent distance (T) exceeds 250', spacing between Type I or II barricades or vertical panels or drums may be increased to 100' within the limits of the tangent, or post mounted delineators at 50' centers may be substituted for barricades, vertical panels or drums.

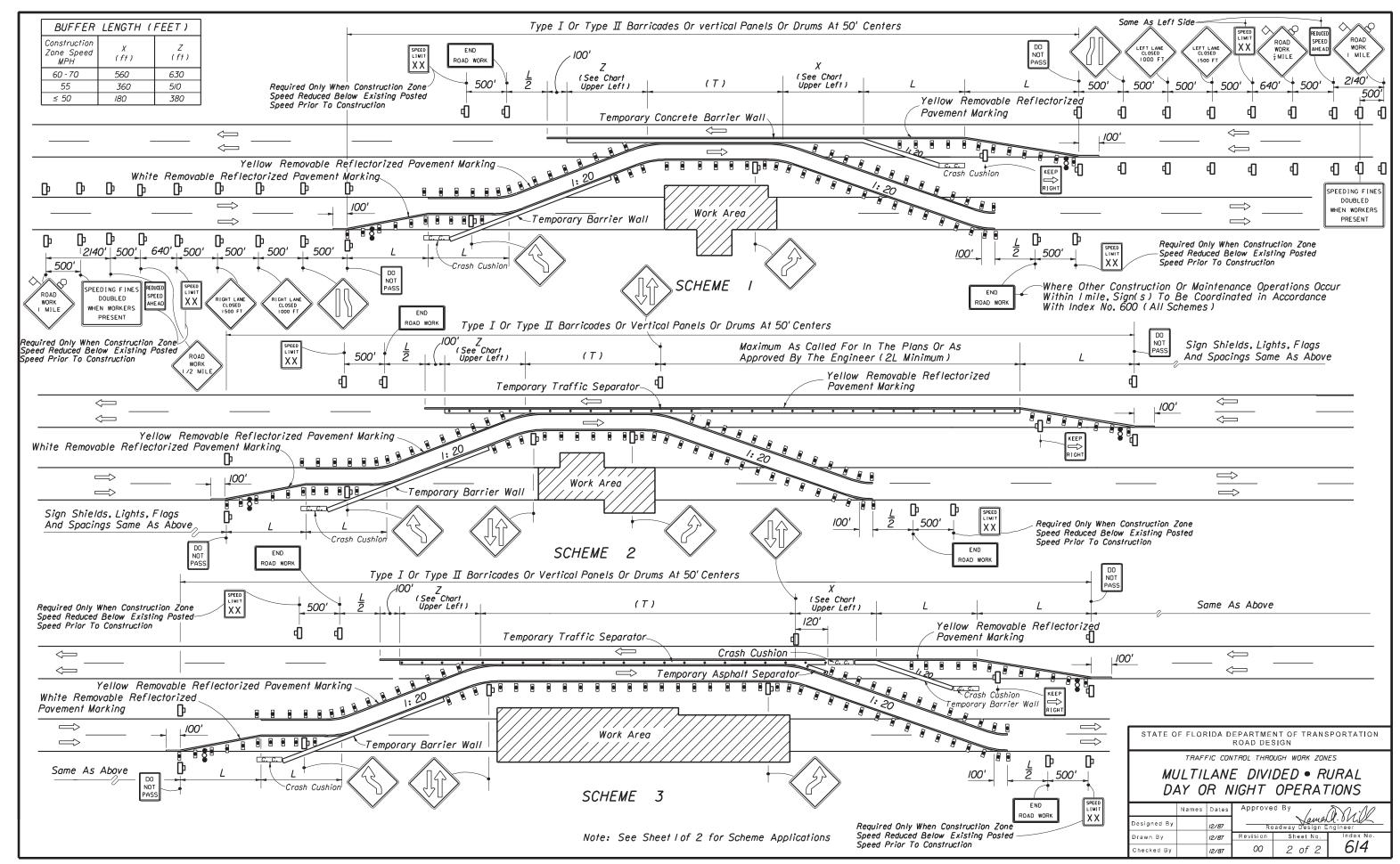
- 7. All existing pavement markings within the realignment which conflict with the revised traffic pattern are to be removed and removable pavement markings used for marking new edge lines.
- 8. Arrows denote direction of traffic only and do not reflect pavement
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. When side roads, cross roads or interchanges are located within the limits for work zone traffic control additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- II. For general TCZ requirements and additional information refer to Index No. 600.
- 12. The contractor has the option of using temporary traffic separators and tubular type warning devices from the qualified products list in lieu of the temporary asphalt traffic separator and tubular warning device detailed above.

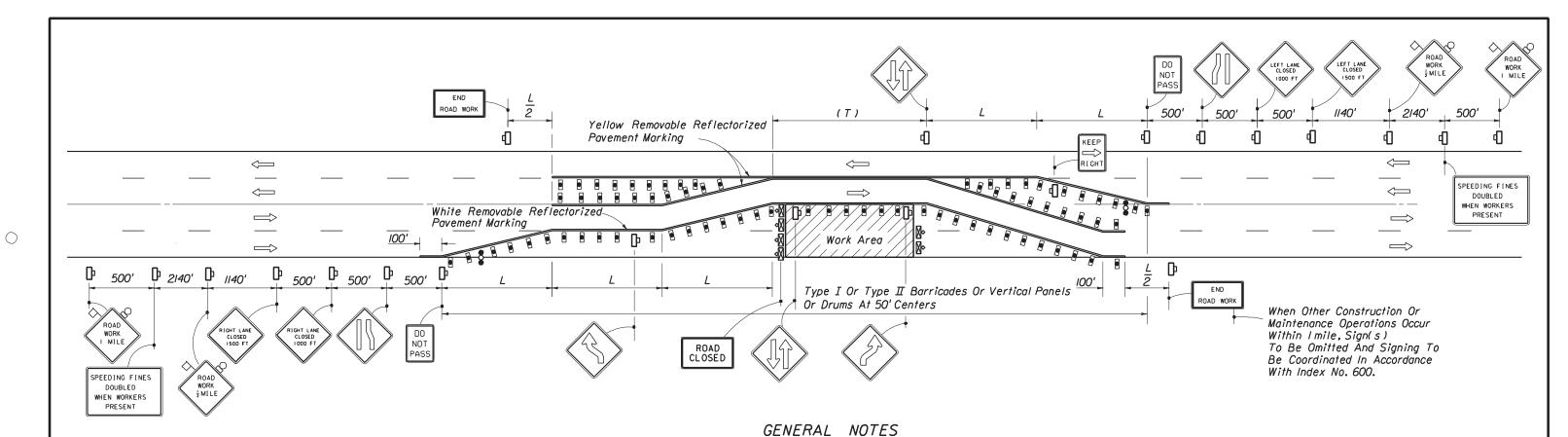
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

MULTILANE DIVIDED . RURAL DAY OR NIGHT OPERATIONS

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Work Area

- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel
 Or Drum (With Steady Burning Light At Night Only).

 (Tubular Markers May Be Used During Daylight Only.
 Cones May Be Used See Index 600).
- ◀ Type III Barricade (With Flashing Light)
- ₩ork Zone Sign
- ••• Advance Warning Arrow Panel

- I. All vehicles, equipment, workers and their activities are restricted at all times to one side of the roadway.
- 2. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.
- 3. All signs, except those required in paved areas, shall be post mounted if the closure time exceeds 12 hours.
- 4. TWO-WAY TRAFFIC signs shall be repeated every $\frac{1}{4}$ mile in each direction, through the tangent distance (T).
- 5. L (min.) = WS for speeds ≥ 45 mph
 - = $\frac{WS^2}{GO}$ for speeds \leq 40 mph

Where:

- W = Width of lateral transition in feet. S = Posted speed limit (mph).
- 6. Where the tangent distance (T) exceeds 250', spacing between cones or tubular markers may be increased to 50' or spacing between Type I or Type II barricades or vertical panels or drums may be increased to 100' within the limits of the tangent.

- 7. This index does not apply when work is being performed in the middle lane(s) of a six or more lane highway. Special maintenance of traffic details will be required.
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- II. For general TCZ requirements and additional information refer to Index No. 600.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT,
WORKERS OR THEIR ACTIVITIES
REQUIRE THE CLOSURE OF THE
LANES IN ONE DIRECTION AND A
DIVERSION IS PROVIDED BY UTILIZING
ONE LANE OF THE OPPOSING
TRAFFIC LANES

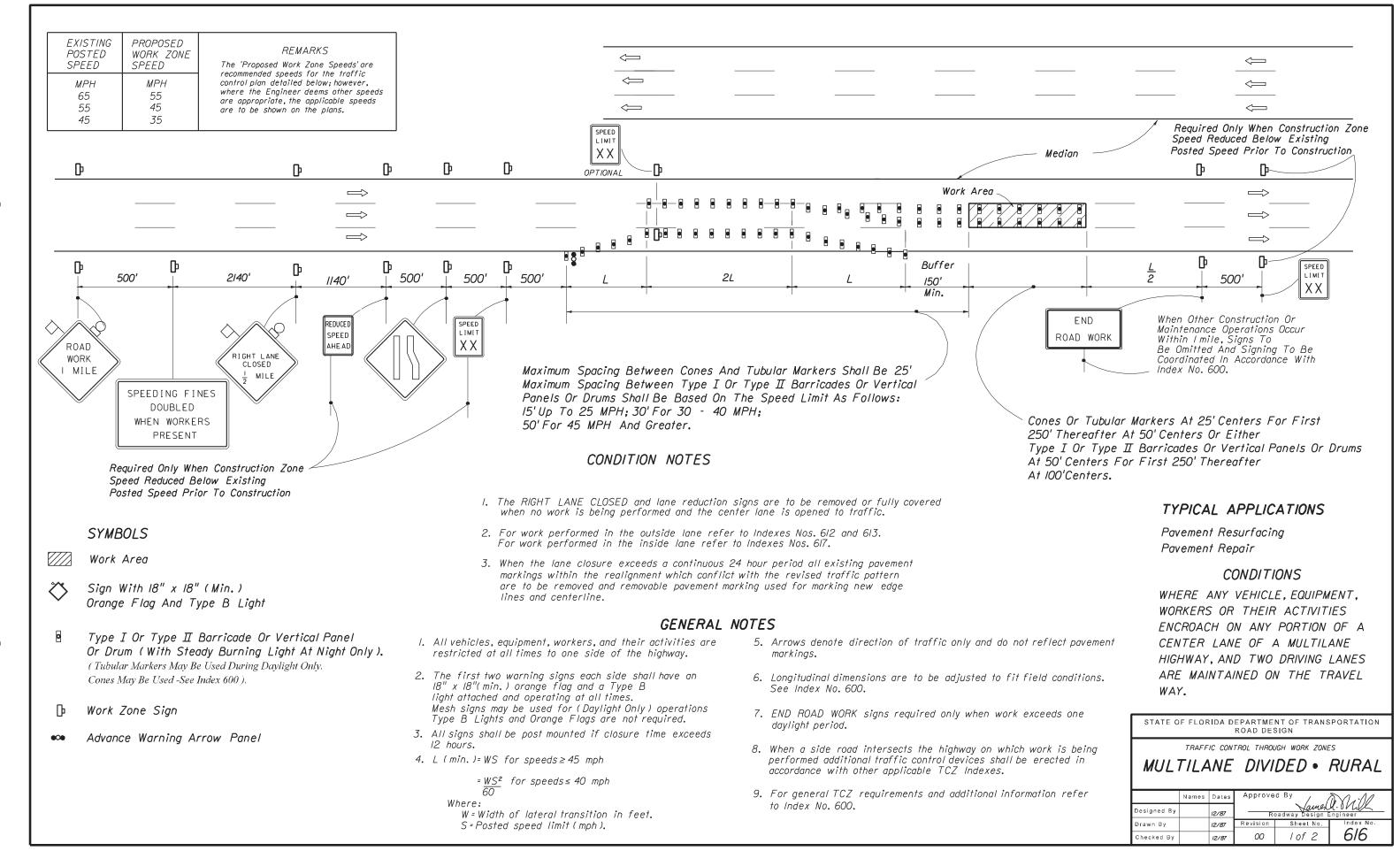
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

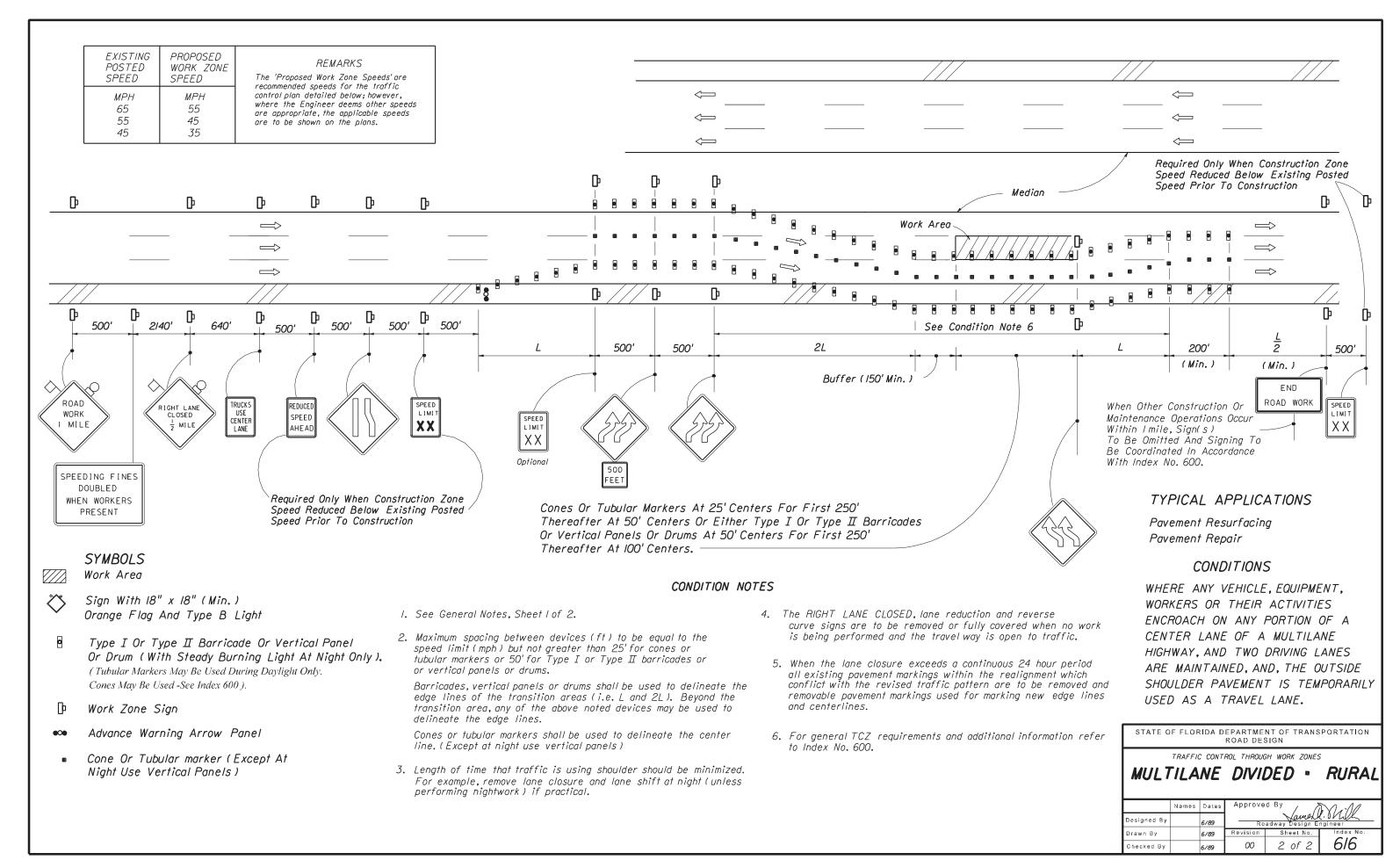
TRAFFIC CONTROL THROUGH WORK ZONES

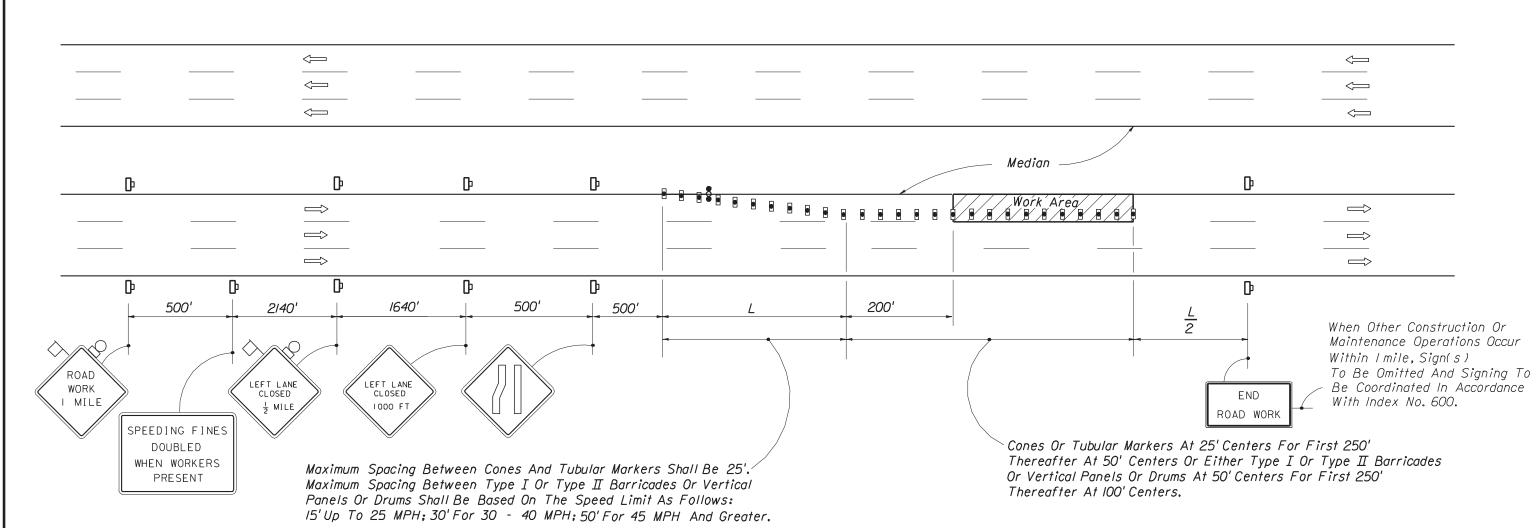
MULTILANE UNDIVIDED ● RURAL DAY OR NIGHT OPERATIONS

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Work Area

Sign With 18" x 18" (Min.)
Orange Flag And Type B Light

- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- ₩ork Zone Sign
- Advance Warning Arrow Panel

GENERAL NOTES

- I. All vehicles, equipment, workers and their activities are restricted at all times to one side of the highway.
- 2. The first two warning signs, each side, shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations
Type B Lights and Orange Flags are not required.

- 3. All signs shall be post mounted if closure time exceeds 12 hours.
- 4. L (min.)= WS for speeds ≥ 45 mph
 - = $\frac{WS^2}{GO}$ for speeds \leq 40 mph

Where:

W = Width of lateral transition in feet S = Posted speed limit (mph).

- 5. The LEFT LANE CLOSED and lane reduction signs are to be removed or fully covered when no work is being performed and the inside lane is open to traffic.
- 6. Advance warning arrow panels are required for both day and night operation. Either the right flashing arrow or the right sequential arrow modes may be used; the caution mode shall not be used.
- 7. Arrows denote direction of traffic only and do not reflect pavement marking.
- 8. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 9. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 10. For work performed in the outside lane refer to Indexes Nos. 612 and 613. For work performed in the center lane refer to Index No. 616
- II. For general TCZ requirements and additional information refer to Index No. 600.

CONDITIONS

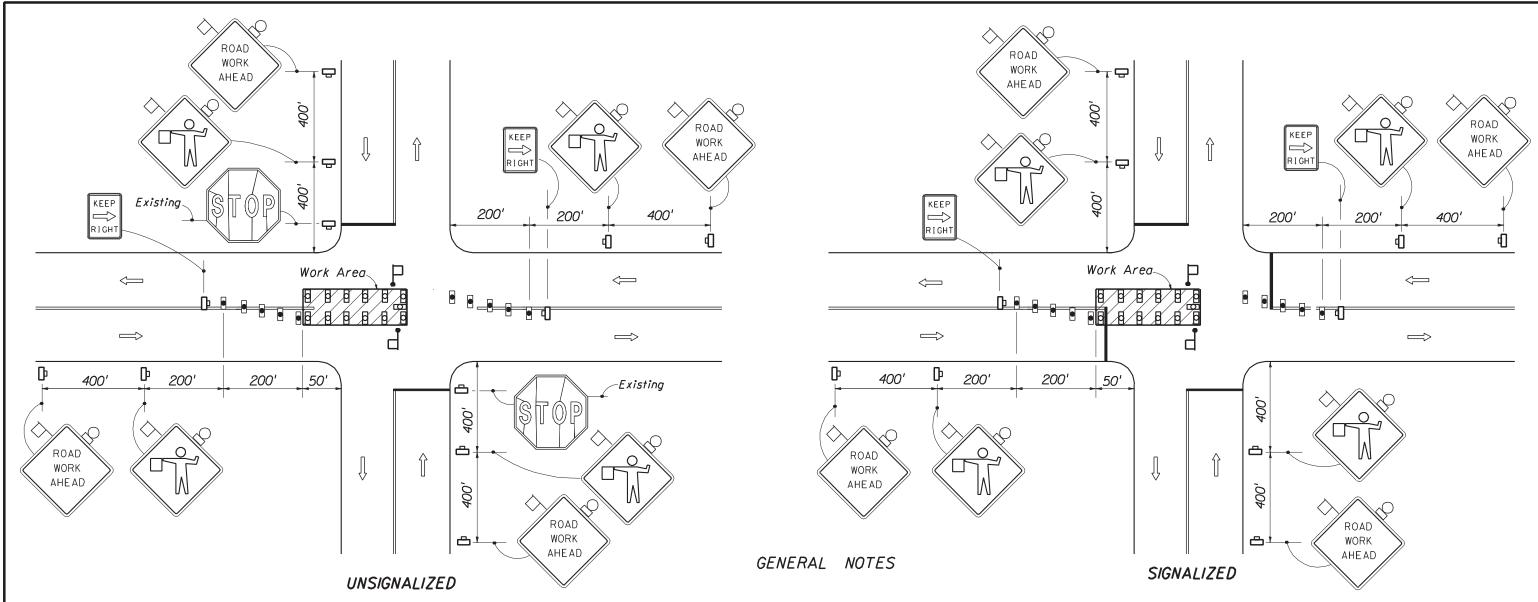
WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES WILL ENCROACH ON ANY PORTION OF THE INSIDE LANE OF A MULTILANE HIGHWAY

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

MULTILANE DIVIDED - RURAL

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Work Area

- Sign With 18" x 18" (Min.)

 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only).

 (Tubular Markers May Be Used During Daylight Only.

 Cones May Be Used -See Index 600).
- Type I Or Type II Barricade Or Vertical
 Panel Or Drum (with Flashing Light At
 Night Only)
- ₩ork Zone Sign
- **□** Flagger
- Stop Bar

- All vehicles, equipment, workers(except flaggers) and their activities are forbidden in lane and intersection areas reserved for traffic.
- 2. The first two warning signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

 Mesh signs may be used for (Daylight Only) operations
 Type B Lights and Orange Flags are not required.
- 3. The FLAGGER legend sign may be substituted for the symbol sign.
- 4. All signs shall be post mounted if closure time exceeds 12 hours.
- 5. When vehicles in a parking zone block the line of sight to TCZ signs or when TCZ signs encroach on a normal pedestrian walkway, the signs shall be post mounted and located in accordance with Index No. 17302.
- 6. Flaggers shall be located where they can control more than one direction of traffic.

Flaggers shall be in sight of each other or in direct communication at all times.

- 7. Maximum spacing between barricades, vertical panels, cones, tubular markers and drums shall be not greater than 25'.
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. Temporary signal phasing modifications are to be approved by the District Traffic Operations Engineer prior to the beginning of work.
- II. Work performed for a period of 60 minutes or less is to be conducted in accordance with Index No. 607.
- 12. For general TCZ requirements and additional information refer to index No. 600.

TYPICAL APPLICATIONS

Utility Work
Pavement Repair

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF A PORTION OF ONE OR MORE TRAFFIC LANES IN AN INTERSECTION FOR A PERIOD OF MORE THAN 60 MINUTES

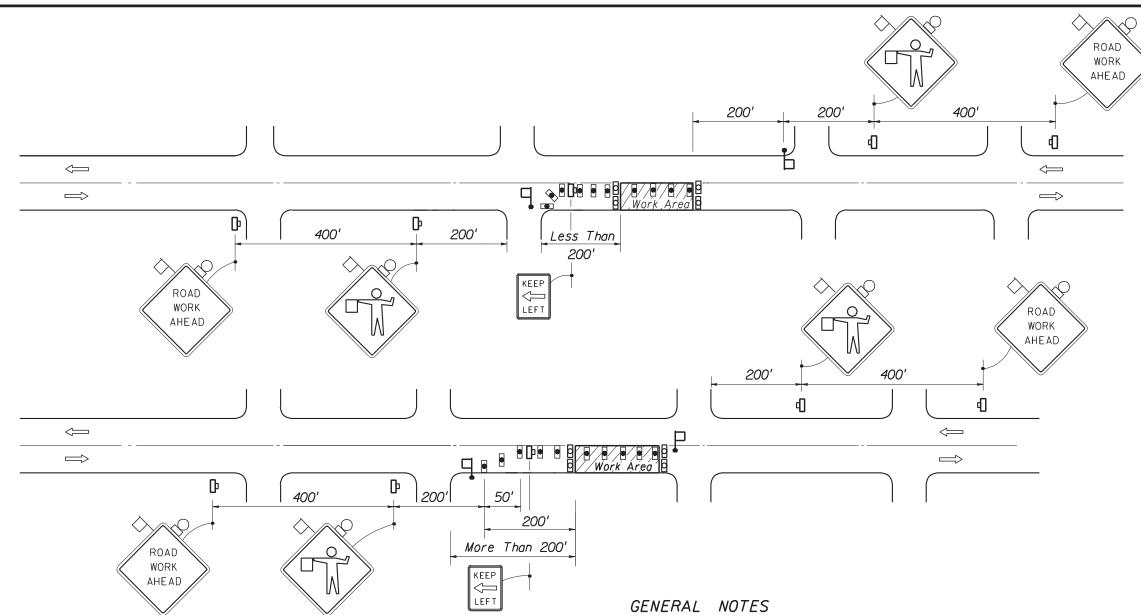
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • URBAN DAY OR NIGHT OPERATIONS

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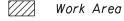
CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS
OR THEIR ACTIVITIES ENCROACH ON THE
PAVEMENT REQUIRING THE CLOSURE OF ONE
TRAFFIC LANE, FOR WORK AREAS LESS THAN
200' DOWNSTREAM FROM AN INTERSECTION
FOR A PERIOD OF MORE THAN 60 MINUTES.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS
OR THEIR ACTIVITIES ENCROACH ON THE
PAVEMENT REQUIRING THE CLOSURE OF ONE
TRAFFIC LANE, FOR WORK AREAS 200'OR
MORE DOWNSTREAM FROM AN INTERSECTION
FOR A PERIOD OF MORE THAN 60 MINUTES.

SYMBOLS



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- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only).

 (Tubular Markers May Be Used During Daylight Only.

 Cones May Be Used -See Index 600).
- Type I Or Type II Barricade Or Vertical Panel Or Drum (with Flashing Light At Night Only)
- ₩ork Zone Sign
- **□** Flagger

- I. Work operations shall be confined to one travel lane, leaving the opposing travel lane open to traffic.
- 2. All vehicles, equipment, workers (except flaggers) and their activities are restricted at all times to one side of the roadway.
- 3. For work operations of 60 minutes or less see Index No. 607
- 4. When vehicles in a parking zone block the line of sight to TCZ signs or when TCZ signs encroach on a normal pedestrian walkway, the signs shall be post mounted and located in accordance with Index No. 17302.
- 5. If work area is confined to an outside auxiliary lane the work area shall be barricaded and the FLAGGER signs replaced by ROAD WORK AHEAD signs. Flaggers are not required.
- 6. Flaggers shall be in sight of each other or in direct communication at all times.

7. The ROAD CONSTRUCTION AHEAD and FLAGGER signs shall have an 18" x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.

- 8. The FLAGGER legend sign may be substituted for the symbol sign.
- 9. All signs shall be post mounted if the closure time exceeds 12 hours.
- 10. The maximum spacing between devices shall be not greater than 25'.
- II. Arrows denote direction of traffic only and do not reflect pavement markings.
- *12.* Longitudinal dimensions are to be adjusted to fit field conditions See Index No. 600.
- 13. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

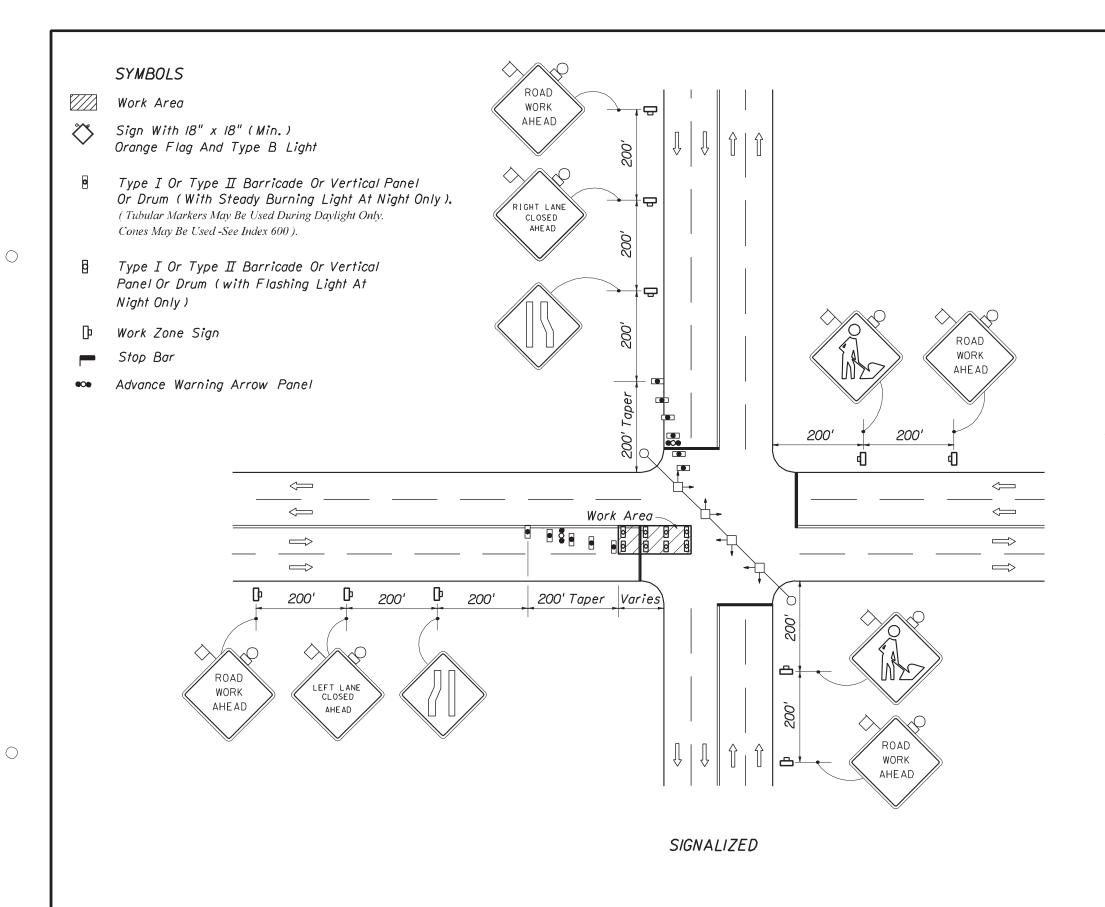
Utility Work
Pavement Repair
Structure Adjustments

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • URBAN DAY OR NIGHT OPERATIONS

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GENERAL NOTES

- All vehicles, equipment, workers (except flaggers) and their activities are forbidden in lane and intersection areas reserved for traffic.
- 2. For work operations of 60 minutes or less see Index No. 607.
- 3. The first two warning signs shall have an I8" x I8" (min.) orange flag and a Type B light attached and operating at all times.

 Mesh signs may be used for (Daylight Only) operations
 Type B Lights and Orange Flags are not required.
- 4. All signs shall be post mounted if closure time exceeds 12 hours.
- 5. The WORKERS legend sign may be substituted for the symbol sign.
- 6. Dual signs are required for divided roadways.
- 7. Arrows denote direction of traffic only and do not reflect pavement markings.
- 8. Maximum spacing between barricades, vertical panels, cones, tubular markers and drums shall be not greater than 25'.
- 9. Temporary signal phasing modifications are to be approved by the District Traffic Operations Engineer prior to the beginning of work.
- 10. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Utility Work
Pavement Repair
Structure Adjustments

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF AT LEAST ONE MEDIAN TRAFFIC LANE FOR A PERIOD OF MORE THAN 60 MINUTES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

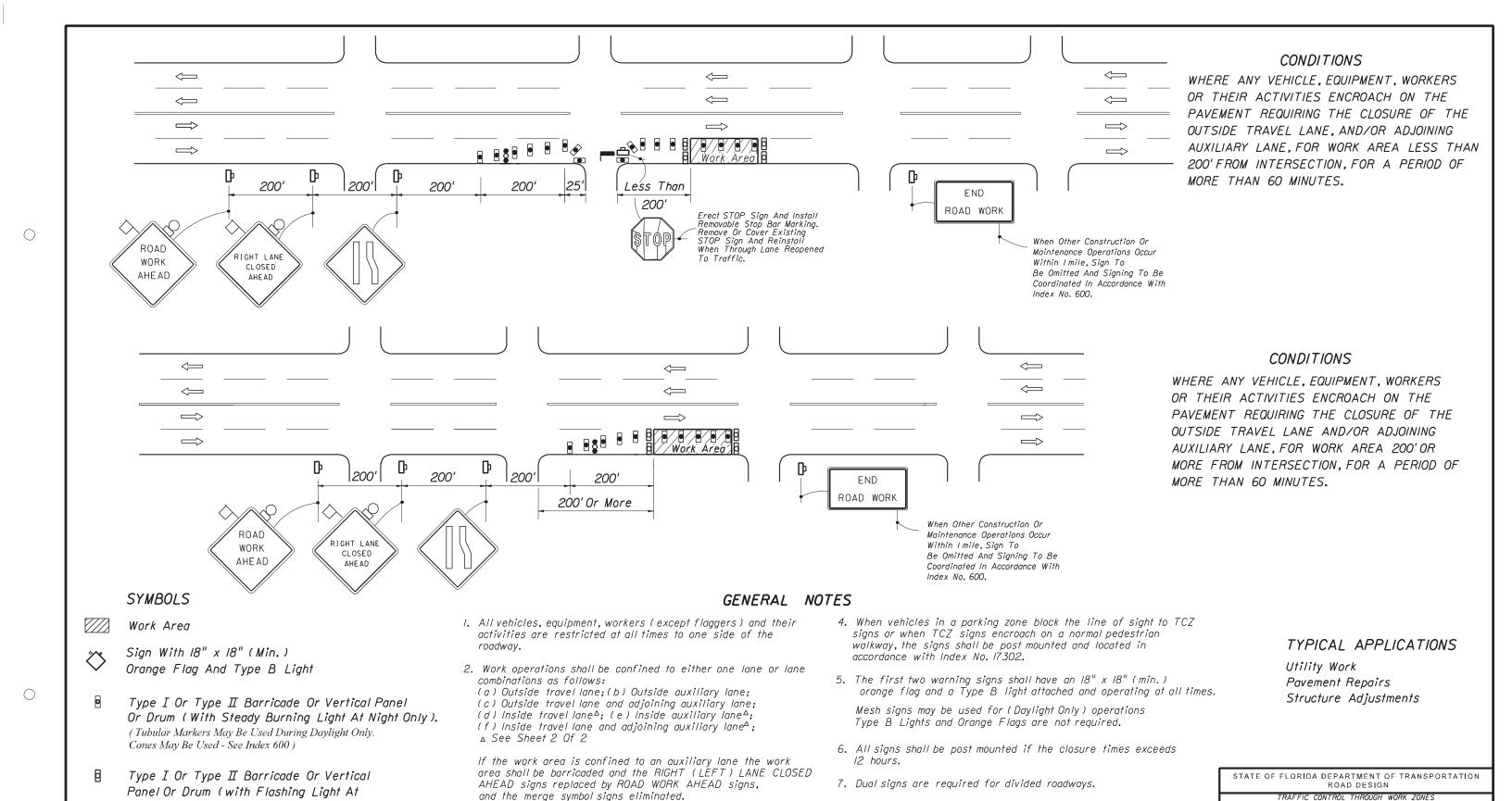
TRAFFIC CONTROL THROIUGH WORK ZONES

MULTILANE, TWO-WAY • URBAN

DIVIDED OR UNDIVIDED

DAY OR NIGHT OPERATIONS

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3. For work operations of 60 minutes or less see Index No. 612.

(Continued)

Night Only)

Stop Bar

Work Zone Sign

Advance Warning Arrow Panel

| 12/87 | Roadway Design Engineer | 12/87 | Revision | Sheet No. | Index No. | 12/87 | 00 | 1 of 2 | 623

MULTILANE. TWO-WAY . URBAN

DIVIDED OR UNDIVIDED

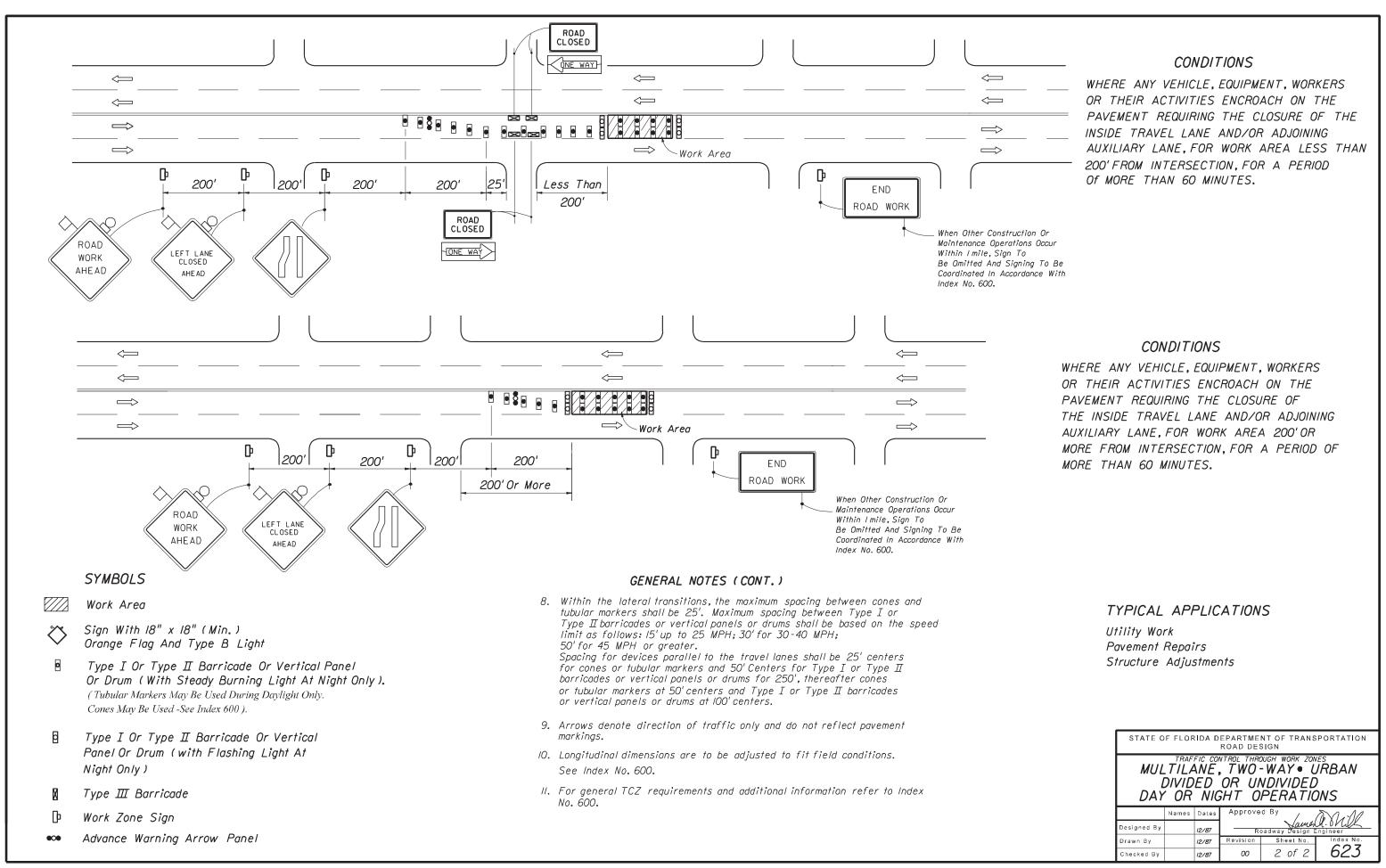
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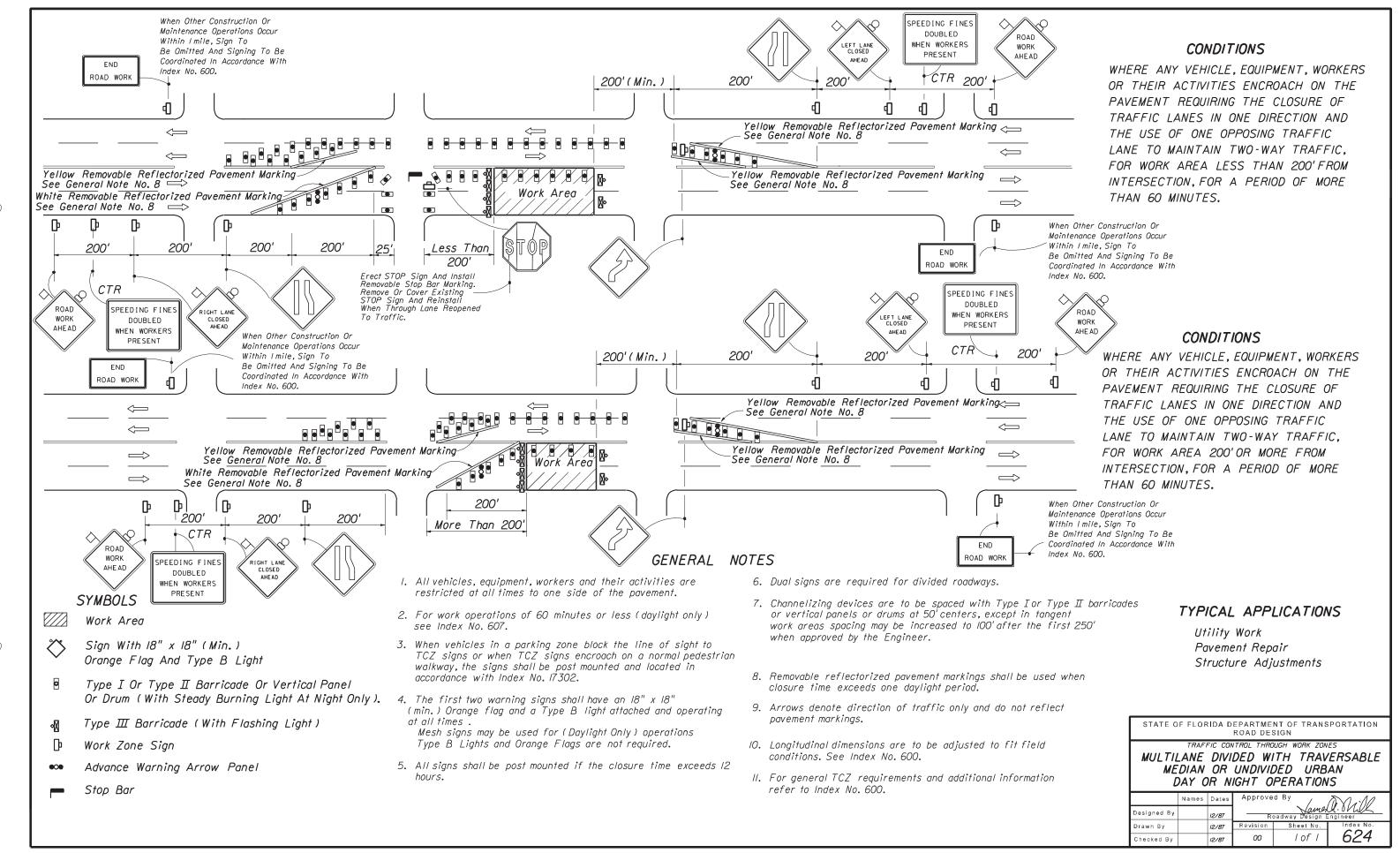
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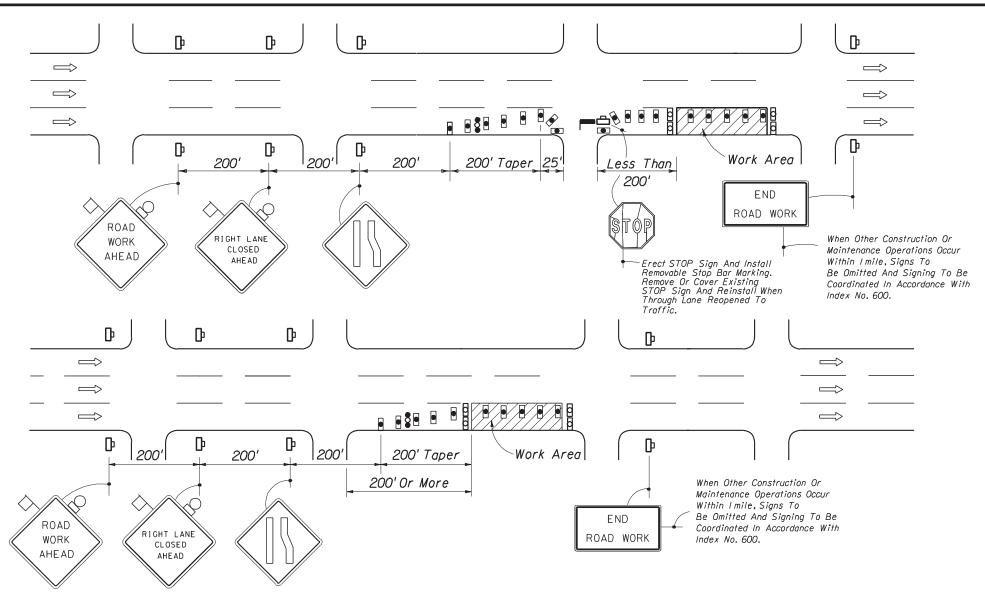
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DAY OR NIGHT OPERATIONS







CONDITIONS

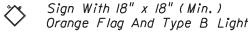
WHERE ANY VEHICLE, EQUIPMENT, WORKERS
OR THEIR ACTIVITIES ENCROACH ON THE
PAVEMENT REQUIRING THE CLOSURE OF EITHER
THE OUTSIDE OR THE MEDIAN TRAVEL LANE
AND/OR ADJOINING AUXILIARY LANE, FOR WORK
AREA LESS THAN 200' FROM INTERSECTION,
FOR A PERIOD OF MORE THAN 60 MINUTES.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS
OR THEIR ACTIVITIES ENCROACH ON THE
PAVEMENT REQUIRING THE CLOSURE OF EITHER
THE OUTSIDE OR THE MEDIAN TRAVEL LANE
AND/OR ADJOINING AUXILIARY LANE, FOR WORK
AREA 200' OR MORE FROM INTERSECTION,
FOR A PERIOD OF MORE THAN 60 MINUTES.

SYMBOLS

Work Area



- Type I Or Type II Barricade Or Vertical Panel
 Or Drum (With Steady Burning Light At Night Only).
 (Tubular Markers May Be Used During Daylight Only.
 Cones May Be Used -See Index 600).
- Type I Or Type II Barricade Or Vertical
 Panel Or Drum (with Flashing Light At
 Night Only)
- ₩ork Zone Sign
- ••• Advance Warning Arrow Panel
- Stop Bar

I. All vehicles, equipment, workers and their activities are

GENERAL NOTES

- restricted at all times to one side of the roadway.
- 2. Work operations shall be confined to either one lane or a combination of lanes as follows:
- (a) Outside travel lane; (b) Outside auxiliary lane;
- (c) Outside travel lane and adjoining auxiliary lane; (d) Outside travel lane and adjoining center lane;
- (e) Outside travel lane and adjoining auxiliary and center lanes;
- (f) Median travel lane $^{\triangle}$; (g) Median auxiliary lane $^{\triangle}$;
- (h) Median travel lane and adjoining auxiliary lane \dot{a} ;
- (i) Median travel lane and adjoining center lane △;
- (j) Median travel lane and adjoining auxiliary and center lanes[△];△See Sheet 2.

If the work area is confined to an auxiliary lane the work area shall be barricaded and the RIGHT LANE CLOSED AHEAD signs replaced by ROAD WORK AHEAD signs and the merge left symbol signs eliminated.

3. For work operations, that require only a single lane closure of 60 minutes or less see Index No. 612.

- 4. When vehicles in a parking zone block the line of sight to TCZ signs or when TCZ signs encroach on a normal pedestrian walkway, the signs shall be post mounted and located in accordance with Index No. 17302.
- 5. When work is performed in the median lane or the median and adjoining center lanes the barricading plans are inverted and LEFT LANE CLOSED AHEAD and merge right symbol signs shall be substituted for the RIGHT LANE CLOSED AHEAD and merge left symbol signs.

If work is confined to the median auxiliary lane the work area shall be barricaded and the LEFT LANE CLOSED AHEAD signs replaced by ROAD WORK AHEAD signs and the merge right symbol signs eliminated.

6. The first two warning signs, each side, shall have an 18"x 18" (min.) orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.

(Continued)

TYPICAL APPLICATIONS

Utility Work Pavement Repair Structure Adjustments

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

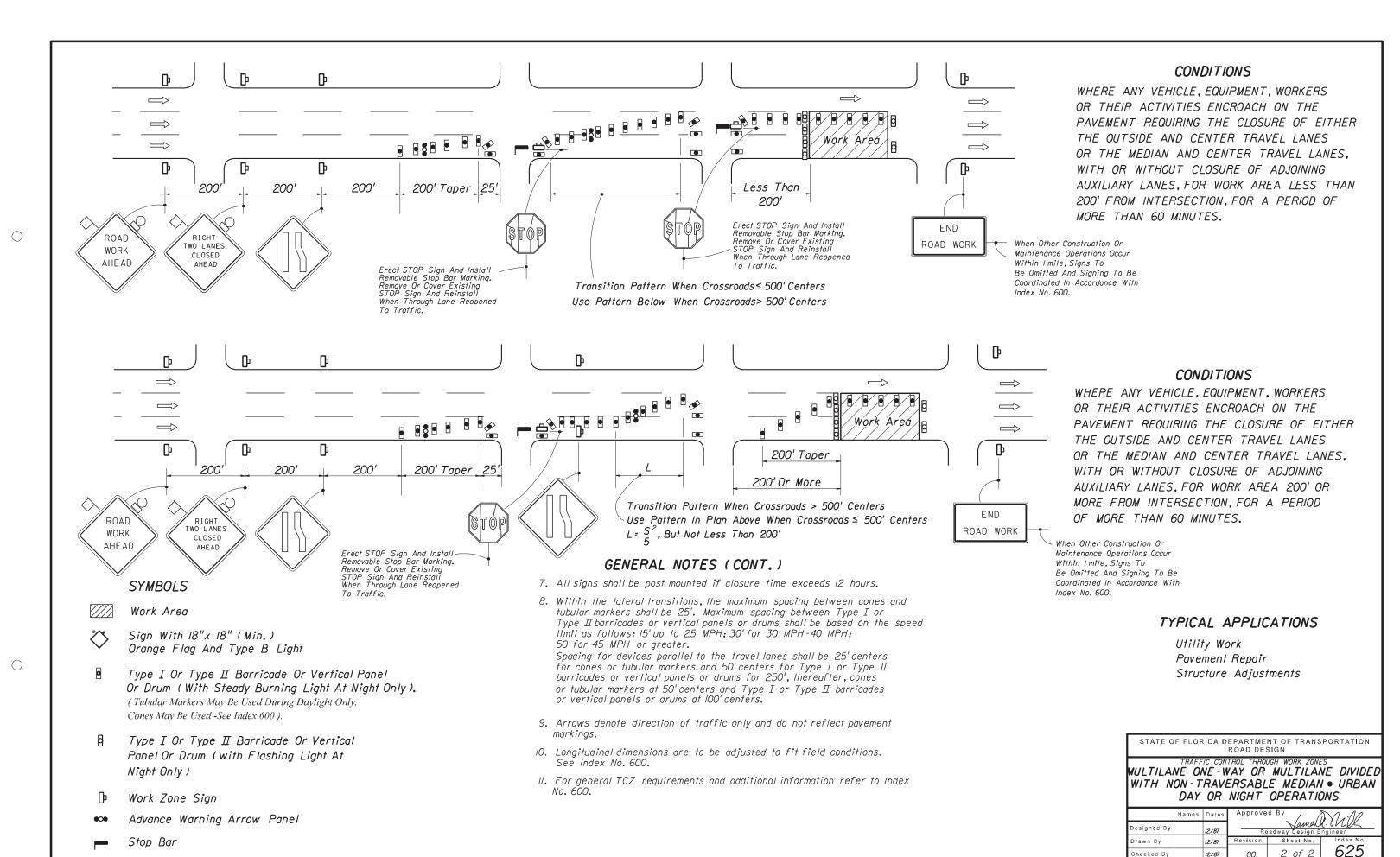
TRAFFIC CONTROL THROUGH WORK ZONES

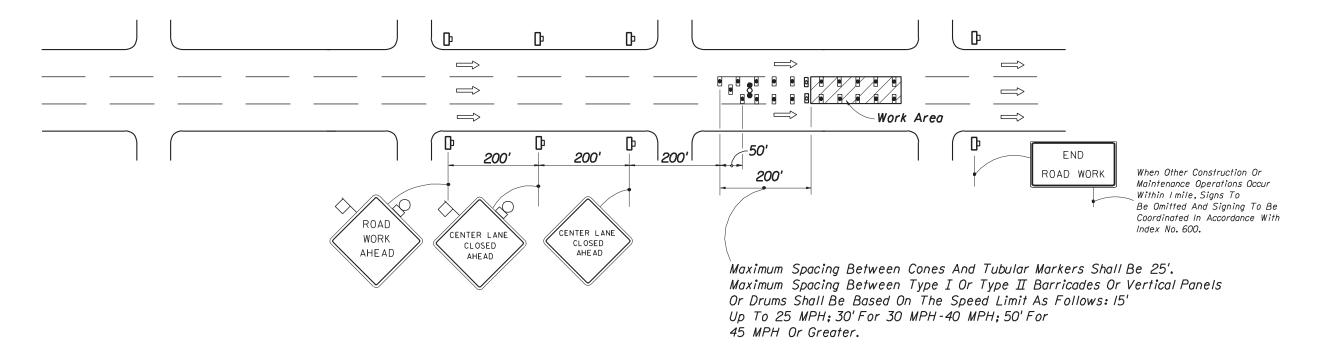
MULTILANE ONE-WAY OR MULTILANE DIVIDED

WITH NON-TRAVERSABLE MEDIAN • URBAN

DAY OR NIGHT OPERATIONS

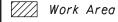
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Drawn By		12/87	Revision	Sheet No.	Index No.
Checked By		12/87	00	1 of 2	625





GENERAL NOTES

SYMBOLS



- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- Type I Or Type II Barricade Or Vertical Panel Or Drum (with Flashing Light At Night Only)
- ₩ork Zone Sign
- Advance Warning Arrow Panel

- All vehicles, equipment, workers and their activities are prohibited at all times from the lane areas reserved for traffic.
- 2. Work operations shall be confined to one center travel lane, leaving the adjacent travel lanes open to traffic.
- 3. For work operations of 60 minutes or less, see Index No. 6/2.
- 4. When vehicles in a parking zone block the line of sight to TCZ signs or when TCZ signs encroach on a normal pedestrian walkway, the signs shall be post mounted and located in accordance with Index No. 17302.
- 5. The first two warning signs, each side, shall have an 18" x 18" orange flag and a Type B light attached and operating at all times.

Mesh signs may be used for (Daylight Only) operations Type B Lights and Orange Flags are not required.

- 6. All signs shall be post mounted if the closure time exceeds 12 hours.
- 7. Advance warning arrow panel is required for both day and night operations.
- 8. Channelizing devices are to be spaced with cones or tubular markers at 25' centers; Type I or Type II barricades or vertical panels or drums at 50' centers for the first 250'; thereafter, cones or tubular markers at 50' centers and Type I or Type II barricades or vertical panels or drums at 100' centers.
- 9. Arrows denote direction of traffic only and do not reflect pavement markings.
- 10. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. For general TCZ requirements and additional information refer to Index No. 600.

TYPICAL APPLICATIONS

Utility Work Pavement Repair Structure Adjustments

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH ON THE PAVEMENT REQUIRING THE CLOSURE OF THE CENTER LANE.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

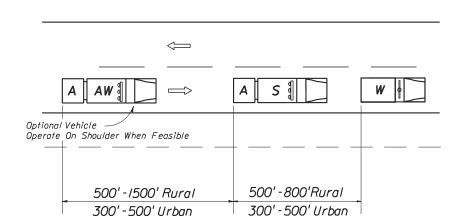
TRAFFIC CONTROL THROUGH WORK ZONES

MULTILANE ONE-WAY OR MULTILANE DIVIDED

WITH NON-TRAVERSABLE MEDIAN • URBAN

DAY OR NIGHT OPERATIONS

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Designed By		12/87	Ro	adway Design E	U. D. W.L. ngineer
Drawn By		12/87	Revision	Sheet No.	Index No.
Checked By		12/87	00	1 of 1	626



MODE • WARNING

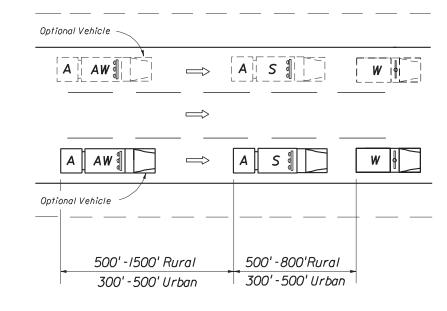
Optional Vehicle A AW AW A S W Optional Vehicle 500'-1500' Rural 300'-500' Urban 300'-500' Urban

MODE • PASS LEFT RIGHT

MOVING OPERATIONS

GENERAL NOTES

- I. These illustrations are representative of general conditions.
- 2. The intensity of light and the position of panels shall be as specified in Index No. 600.
- 3. The Advance Warning Vehicle (Optional) may be used at the direction of the Engineer. If an Advance Warning Vehicle is operated within the travel way, an approved Truck Mounted Attenuator will be required on the Advance Warning Vehicle but not required on the Shadow Vehicle. The Advance Warning Arrow Panel and Warning Sign are required on both the Advance Warning and Shadow Vehicles.
- 4. For general TCZ requirements and additional information refer to Index No. 600.
- 5. If the work vehicle speed exceeds the minimum legal speed limit on limited access facilities and one half the posted speed limit on other facilities the engineer in charge may delete requirements for shadow vehicle and attenuators. The work vehicle will be required to have an advance warning arrow panel and sign message



MODE • PASS LEFT RIGHT

TYPICAL APPLICATIONS

Striping RPM Placement Vegetation Control

CONDITIONS
MOVING OPERATION

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

MOVING OPERATIONS

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signed By		12/87	Roadway Design Engineer				
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SYMBOLS

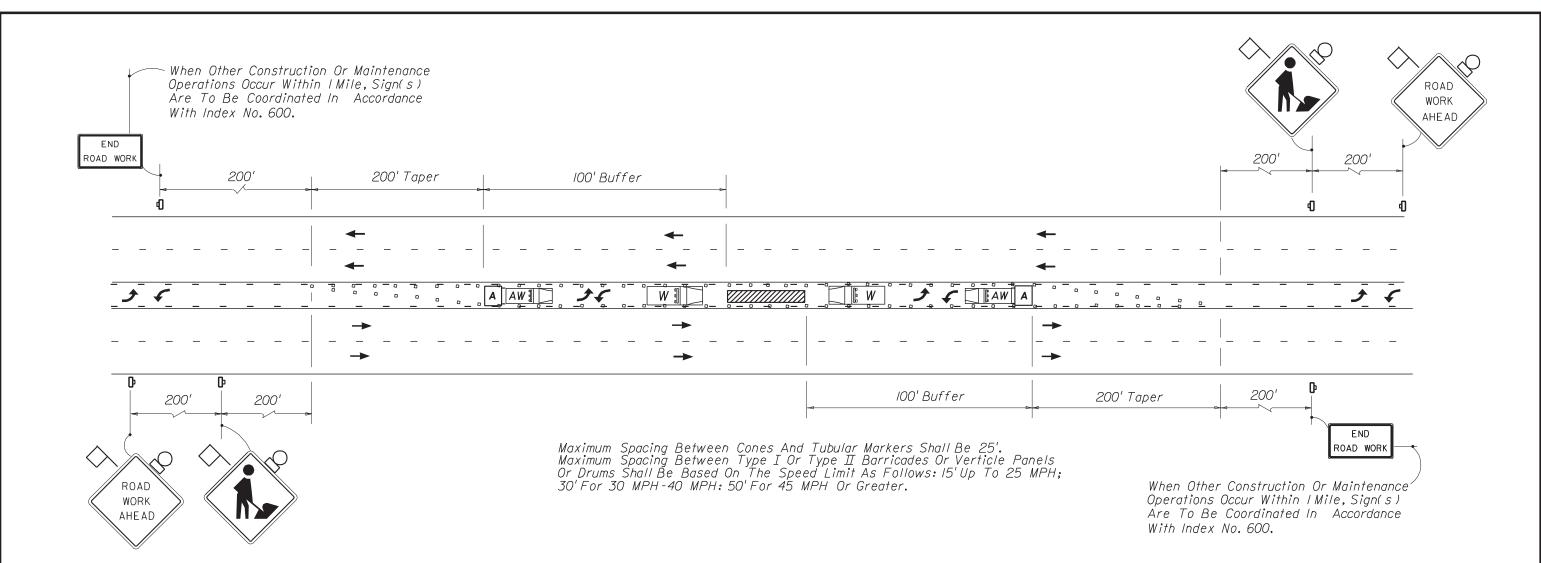
WW Work Vehicle With Flashing Beacon

 \bigcirc

Shadow (S) Or Advance Warning (AW) Vehicle
With Advance Warning Arrow Panel And Sign Message

Truck Mounted Attenuator (TMA)

> Lane Identification And Direction Of Traffic



GENERAL NOTES

- I. Work operations shall be confined to two way left turn lane, leaving the adjacent lanes open to traffic.
- 2. The first two warning signs, each side, shall have an 18" x 18" (min.) Orange Flag and a Type B light attached and operating at all times. Mesh signs may be used for (Daylight Only) operations, Type B Light's and Orange Flags are not Required.
- 3. Advance Warning Vehicle will have an Advanced Warning Arrow Panel in the Warning Mode.
- 4. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 5. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 6. For general TCZ requirements and additional information, refer to Index No. 600.

TYPICAL APPLICATIONS

Pavement Repair Utility Work

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ARE BEING CONDUCTED IN THE TWO WAY LEFT TURN LANE.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

TWO WAY LEFT TURN LANE CLOSURE

Approved By Names Dates Designed By rawn By 628 hecked By 00 I of I

SYMBOLS

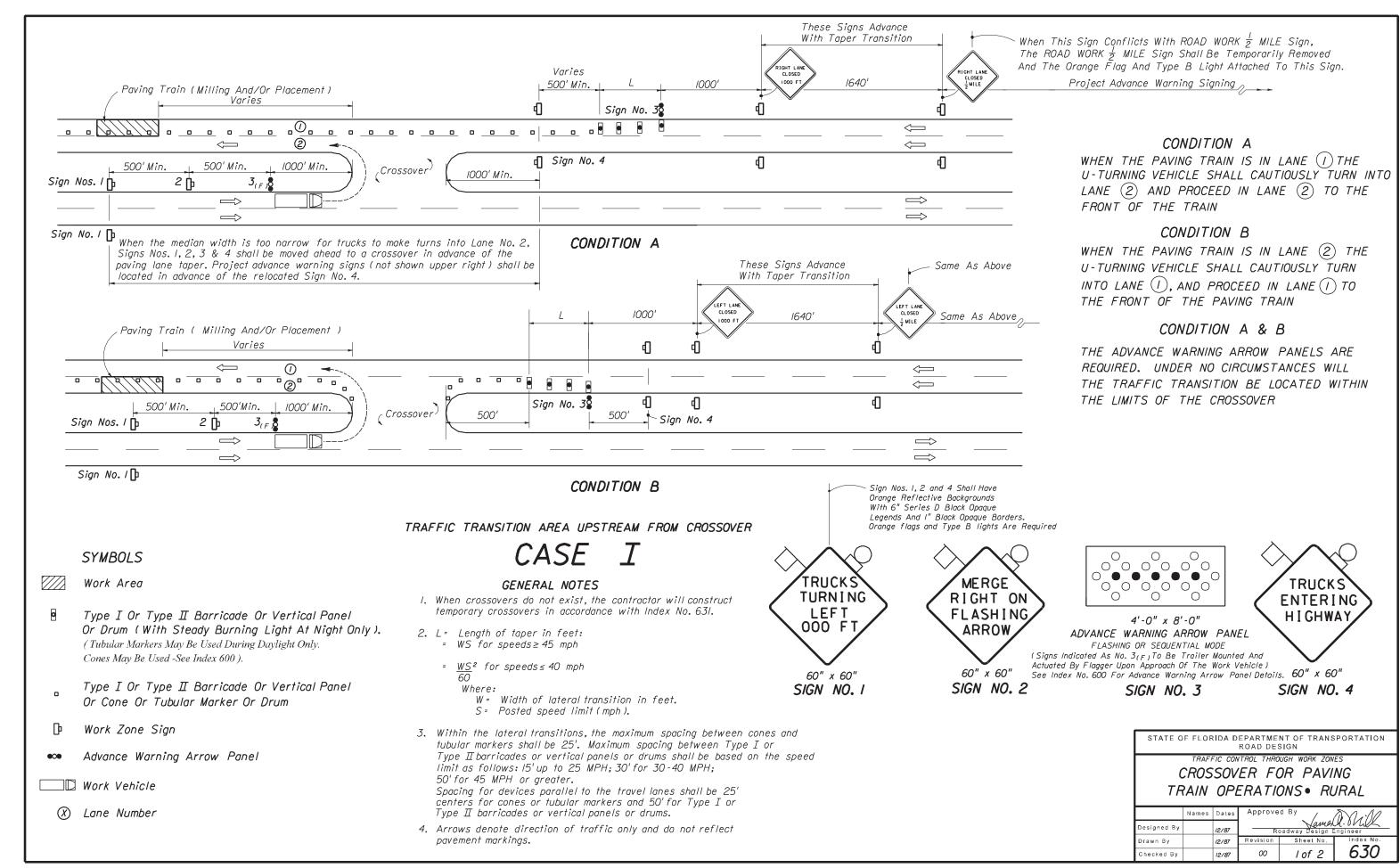
Work Area

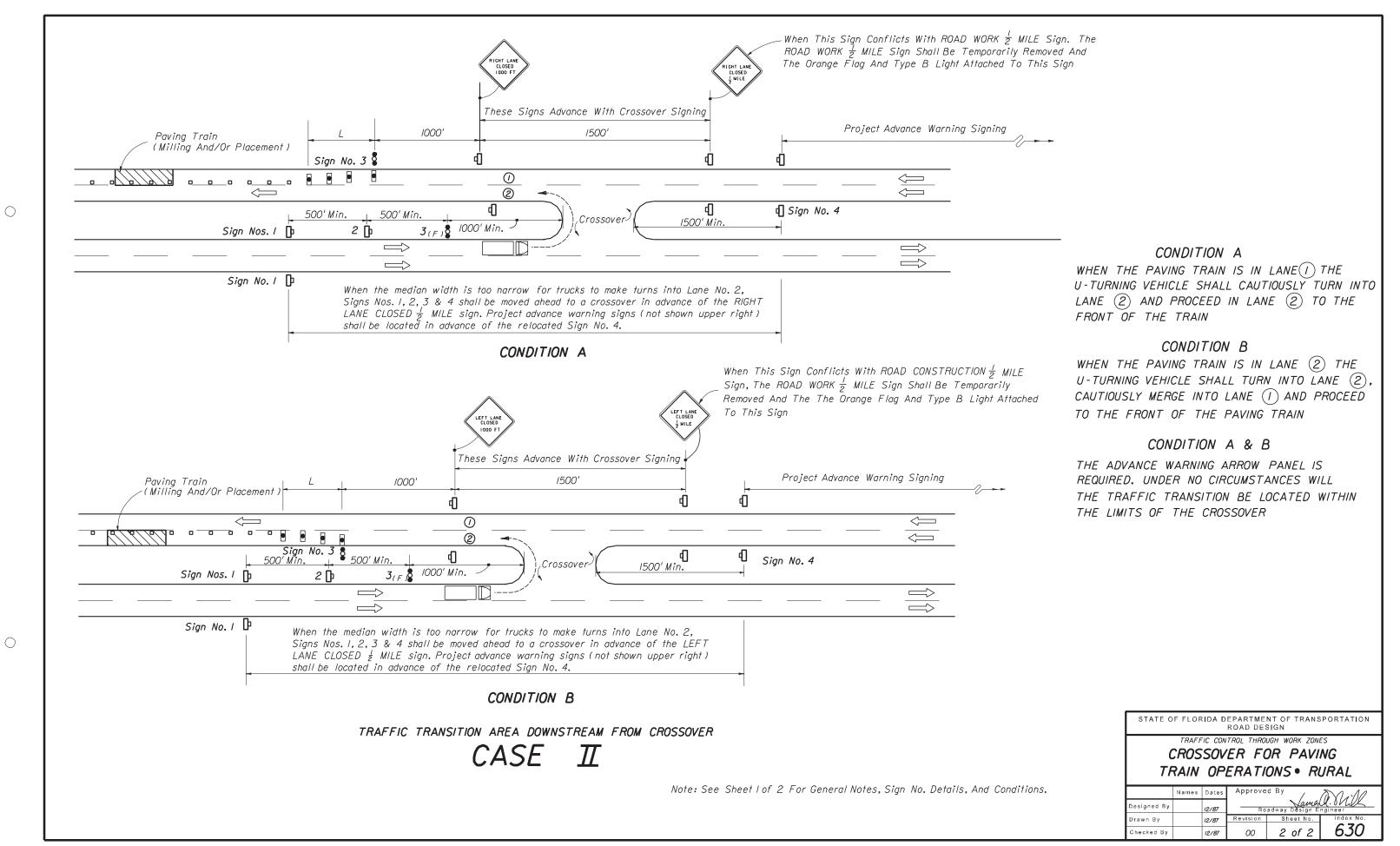
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burn Light At Night Only)
- Type I Or Type II Barricade Or Verticle Panel Or Cone Or Tubular Marker Or Drum
- ₩ork Zone Sign

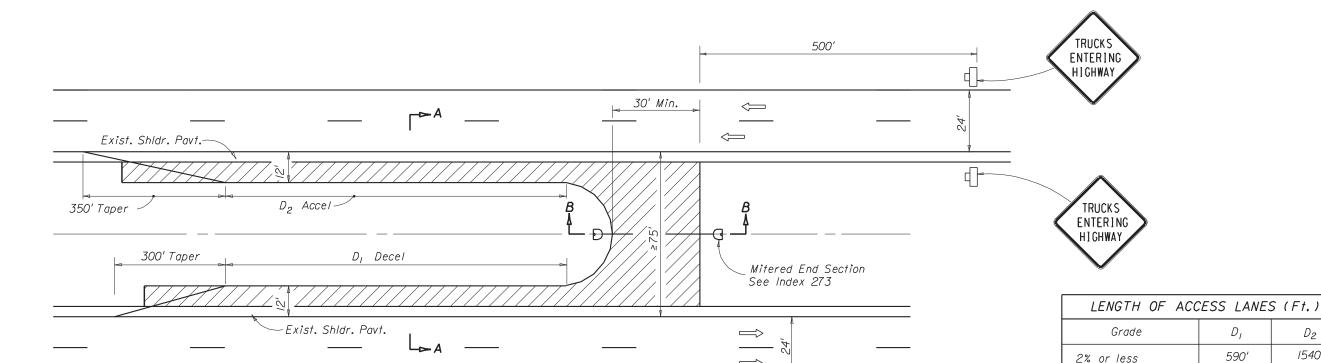
W | Work Vehicle With Flashing Beacon (optional)



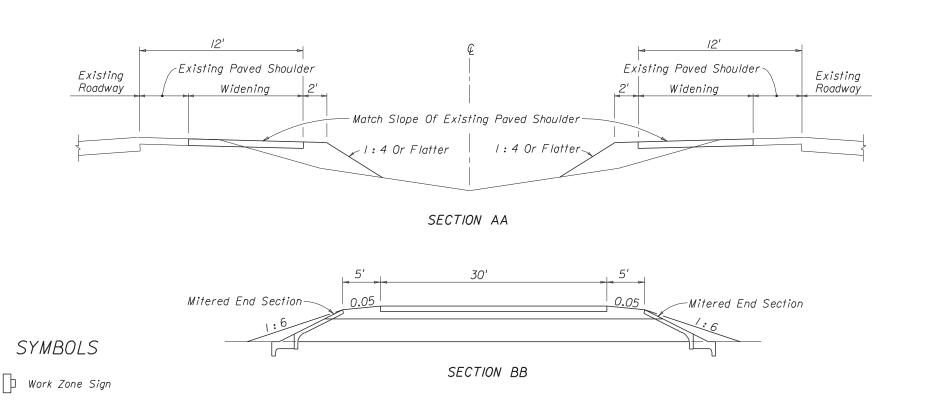
Advance Warning Vehicle Equipped With Advance Warning Arrow Panel And Truck Mounted Attenuator







PLAN



TEMPORARY CROSSOVER FOR MEDIAN WIDTHS ≥ 75'

 \Longrightarrow

GENERAL NOTES

3 to 4% Upgrade

3 to 4% Downgrade

I. Temporary median crossovers shall be within the project limits and shall not be used for transporting materials to or from any other project. The acceleration-deceleration surfaces shall be paved. RAP material is acceptable for crossing surfacing.

 D_I

590'

530'

7/0'

1540'

2310'

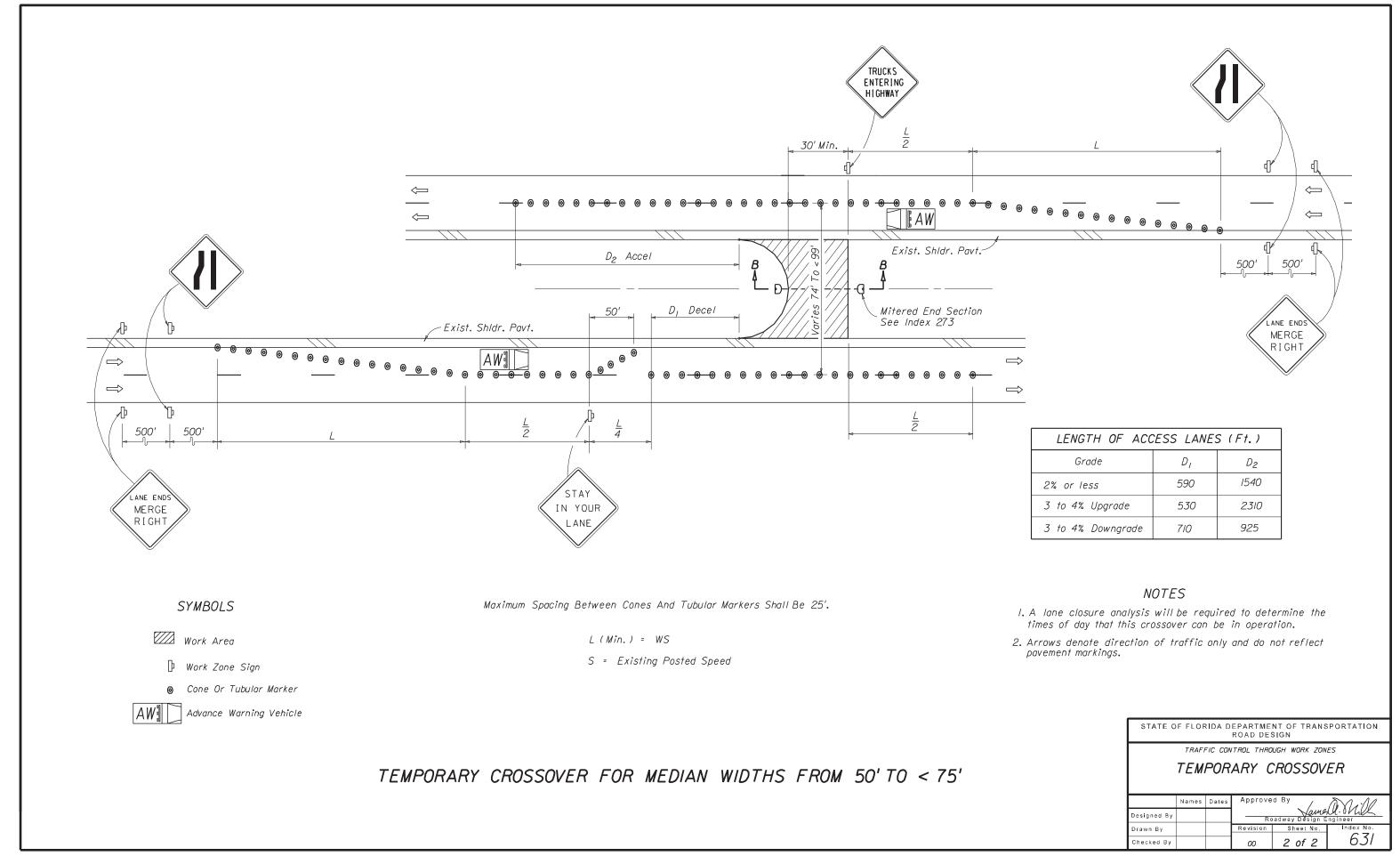
925'

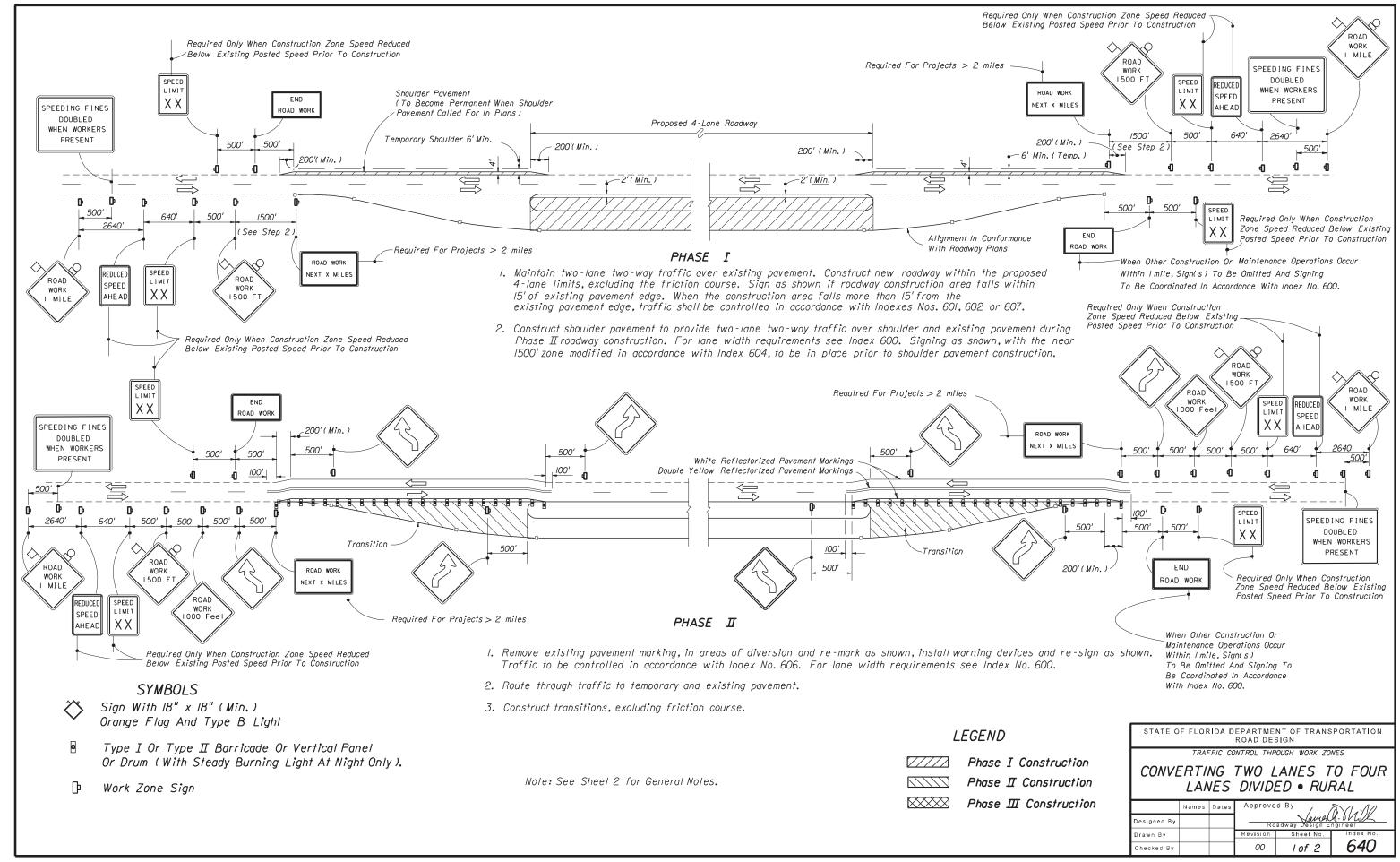
- 2. Temporary median crossovers shall be located only in areas having adequate sight distance. On limited access facilities temporary median crossovers shall not be located within 1.5 miles of interchanges nor within 2000 ft. of acceleration-deceleration lanes at rest areas, other access openings or other highway service areas.
- 3. For paving train operations at permanent crossovers, see Index 630.
- 4. All traffic control devices are to be removed when crossover will not be in use for one hour or longer.
- 5. Trailer mounted advance warning panel may be used in lieu of advance warning vehicle.
- 6. When a crossover is no longer needed, all temporary construction shall be immediately removed and the area restored to its original condition.
- 7. Cost of construction, maintenance, removal and restoration work related to temporary crossovers shall be included in the contract unit price for Maintenance of Traffic, LS.

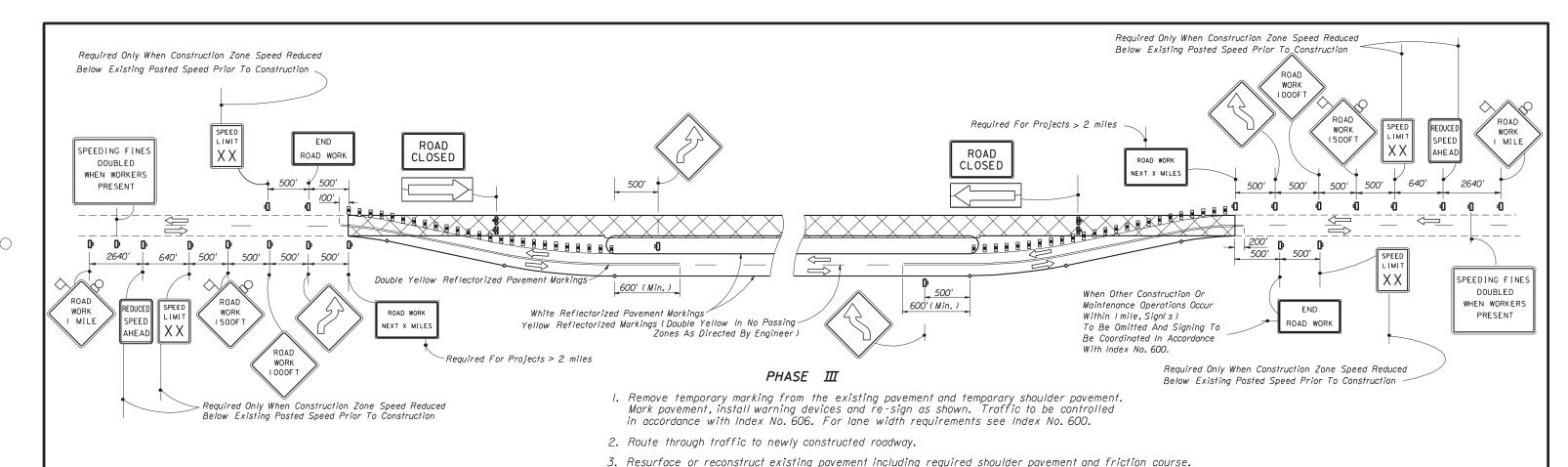
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN TRAFFIC CONTROL THROUGH WORK ZONES

TEMPORARY CROSSOVER

	Names	Dates	Approve	d By	Mill
Designed By			Ro	adway Design E	
Drawn By			Revision	Sheet No.	Index No.
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PHASE IV

- I. Reroute through traffic as shown in Phase II. Signing to be as shown in Phase II.
- 2. Construct friction course over pavement constructed in Phases I and ${\rm I\!I}$.

GENERAL NOTES

- SYMBOLS
- Sign With 18" x 18" (Min.) Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- Type III Barricade (With Flashing Light)
- ₩ork Zone Sign

- I. The first two warning signs shall have an I8" x I8" (min.) orange flag And a Type B light attached and operating at all times.
- 2. Existing signs and pavement markings that conflict with construction signing and marking shall be obliterated or removed.
- 3. Lane widths for maintenance of two-way traffic should desirably be equal to lane widths of the existing facility, but lanes shall be not less than 10' in width. When one-lane one-way operations are necessary, a minimum width of 12' shall be maintained and traffic controlled in accordance with Indexes Nos. 603, 604, 606 and 607. Minimum width for the temporary shoulders is 6'
- 4. Within the lateral transitions, the maximum spacing between Type I or Type II barricades or vertical panels or drums shall be based on the speed limit as follows: 15'up to 25 MPH; 30' for 30-40 MPH; 50' for 45 MPH or greater.

The maximum spacing between warning devices used for delineation between the travel way and construction area to be 50' for Type I or Type II barricades or vertical panels or drums.

5. Warning Devices shall be in conformance with 'DropOffs In Work Zones'. Index No. 600.

- 6. For speed sign applications, see 'Regulatory Speed In Work Zones 'Index No. 600.
- 7. For reflectorized raised pavement marker applications, see Reflective Pavement Markers Index 600 and Index No 17352.
- Additional barricades, signing, lighting or other traffic controls shall be provided for limited work areas in accordance with other applicable TCZ Indexes.
- Arrows denote direction of traffic only and do not reflect pavement markings.
- IO. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- II. When a side road intersects the highway on which work is being performed additional traffic control devices shall be erected in accordance with other applicable TCZ Indexes.
- 12. Provisions approved by the Engineer shall be made for the removal of storm water from the roadway(s) during construction.
- For general TCZ requirements and additional information refer to Index No. 600.

LEGEND

Phase I Construction

Phase II Construction

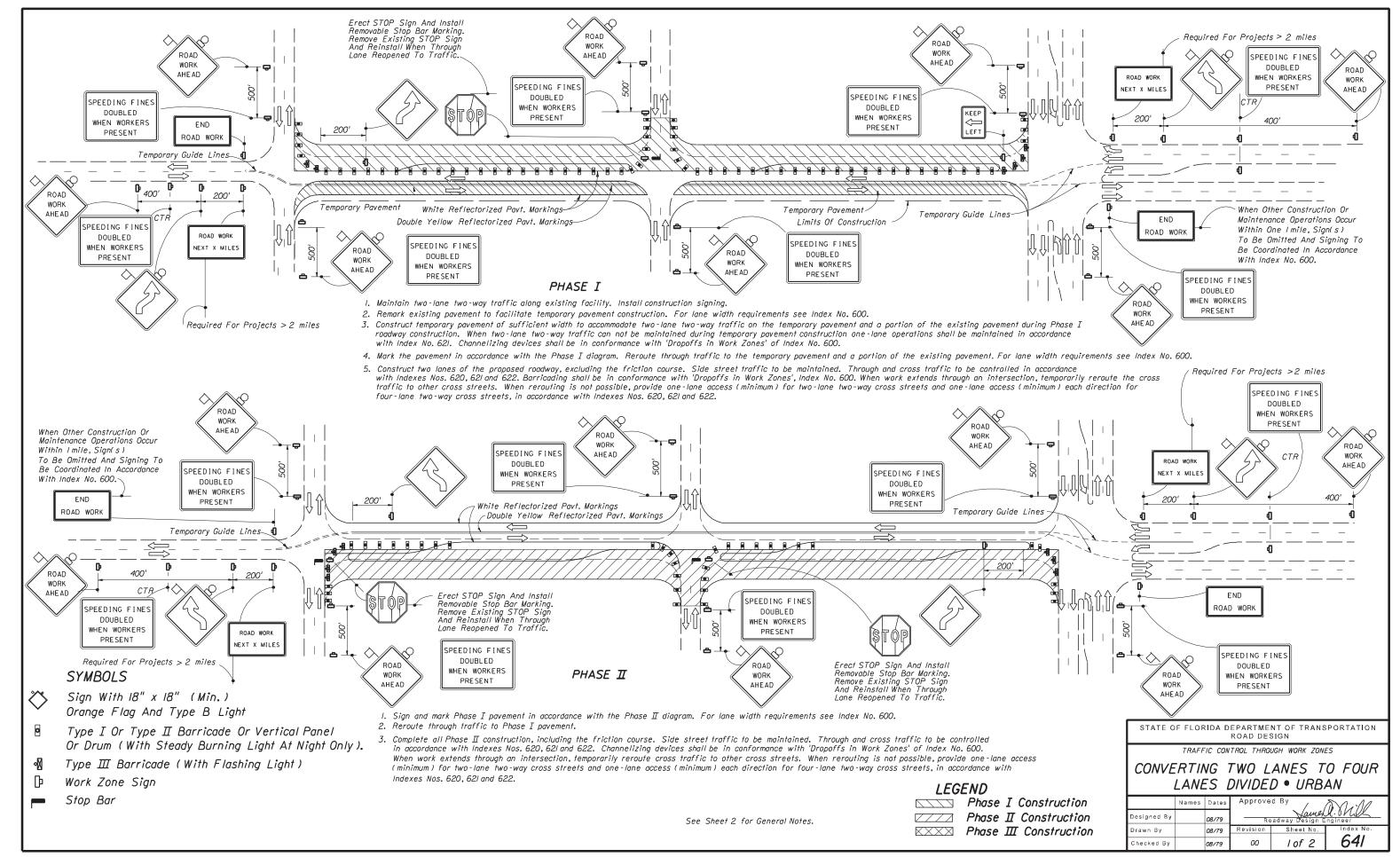
Phase III Construction

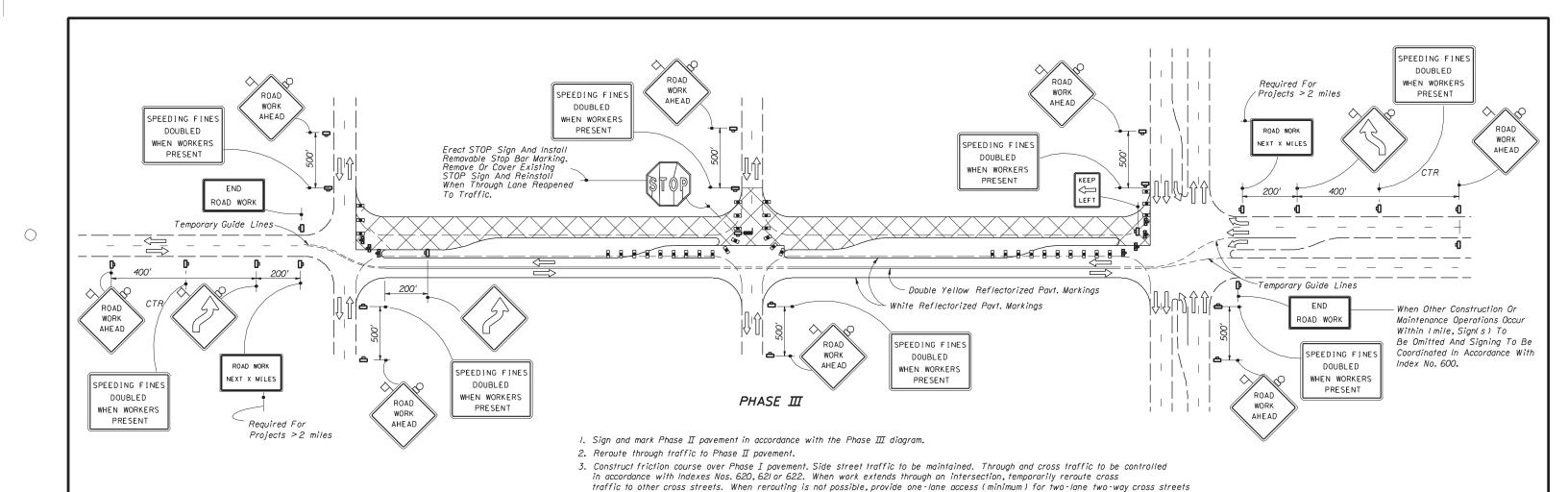
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

CONVERTING TWO LANES TO FOUR LANES DIVIDED • RURAL

	Names	Dates	Approve		To Carll
Designed By			Ro	adway Design E	ingineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	2 of 2	640





- Sign With 18" x 18" (Min.)
 Orange Flag And Type B Light
- Type I Or Type II Barricade Or Vertical Panel
 Or Drum (With Steady Burning Light At Night Only).
 Tubular Markers May Be Used During Daylight Only.
- √ Type Ⅲ Barricade (With Flashing Light)
- ₩ork Zone Sign
- Stop Bar

GENERAL NOTES

- I. All signing, pavement marking, barricades and warning lights necessary for maintenance of traffic shall conform to Index No. 600.
- 2. The first two warning signs shall have an I8" x I8" (min.) orange flag and a Type B light attached and operating at all times.
- 3. Lane widths for maintenance of two-way traffic should desirably be equal to lane widths of the existing facility, but lanes shall not be less than 10' in width. When one-lane one-way operations are necessary, a minimum width of 12' should be maintained and traffic controlled in accordance with Indexes Nos. 620, 621 or 622.
- 4. At signalized intersections, signals shall be directed or relocated as required to the center of relocated lanes.
- 5. For reflectorized raised payment marker application see index No. 600 and index No. 17352.

and one-lane access (minimum) each direction for four-lane two-way cross streets.

- 6. Additional barricades, signing, lighting or other traffic controls for limited work areas shall be provided in accordance with other applicable TCZ Indexes as conditions warrant in each phase.
- $7. \ \textit{Provisions approved by the Engineer shall be made for the removal of storm water from the roadway(s) during construction.}$
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. For general TCZ requirements and additional information refer to Index No. 600.

LEGEND

Phase I Construction

/// Phase II Construction

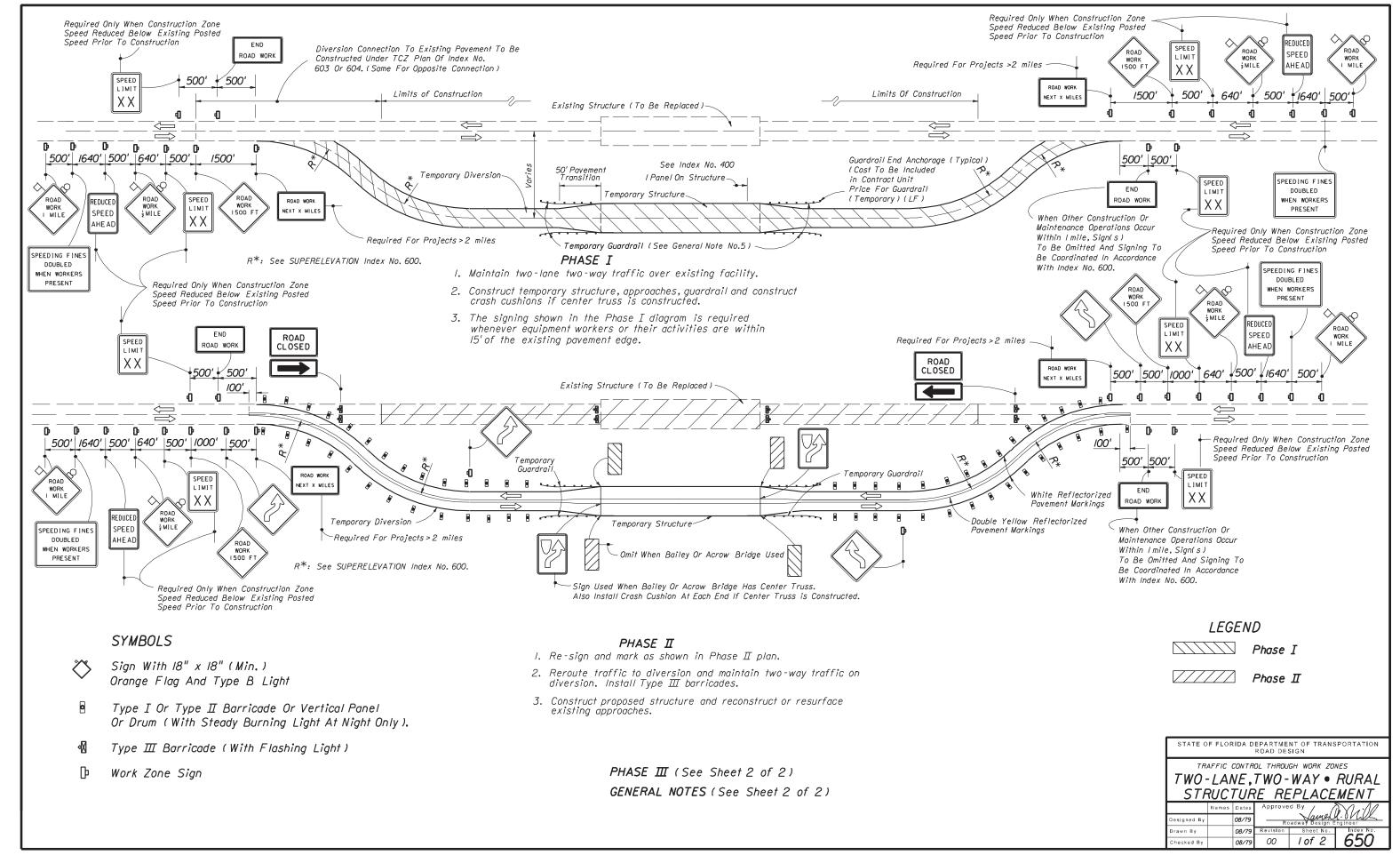
XXXX Phase III Construction

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL THROUGH WORK ZONES

CONVERTING TWO LANES TO FOUR LANES DIVIDED • URBAN

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Designed By		08/79	Roadway Design Engineer			
Drawn By		08/79	Revision	Sheet No.	Index No.	
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PHASE III

- I. Reroute traffic to final alignment and maintain two-way traffic.
- 2. Remove all temporary construction items.

GENERAL NOTES

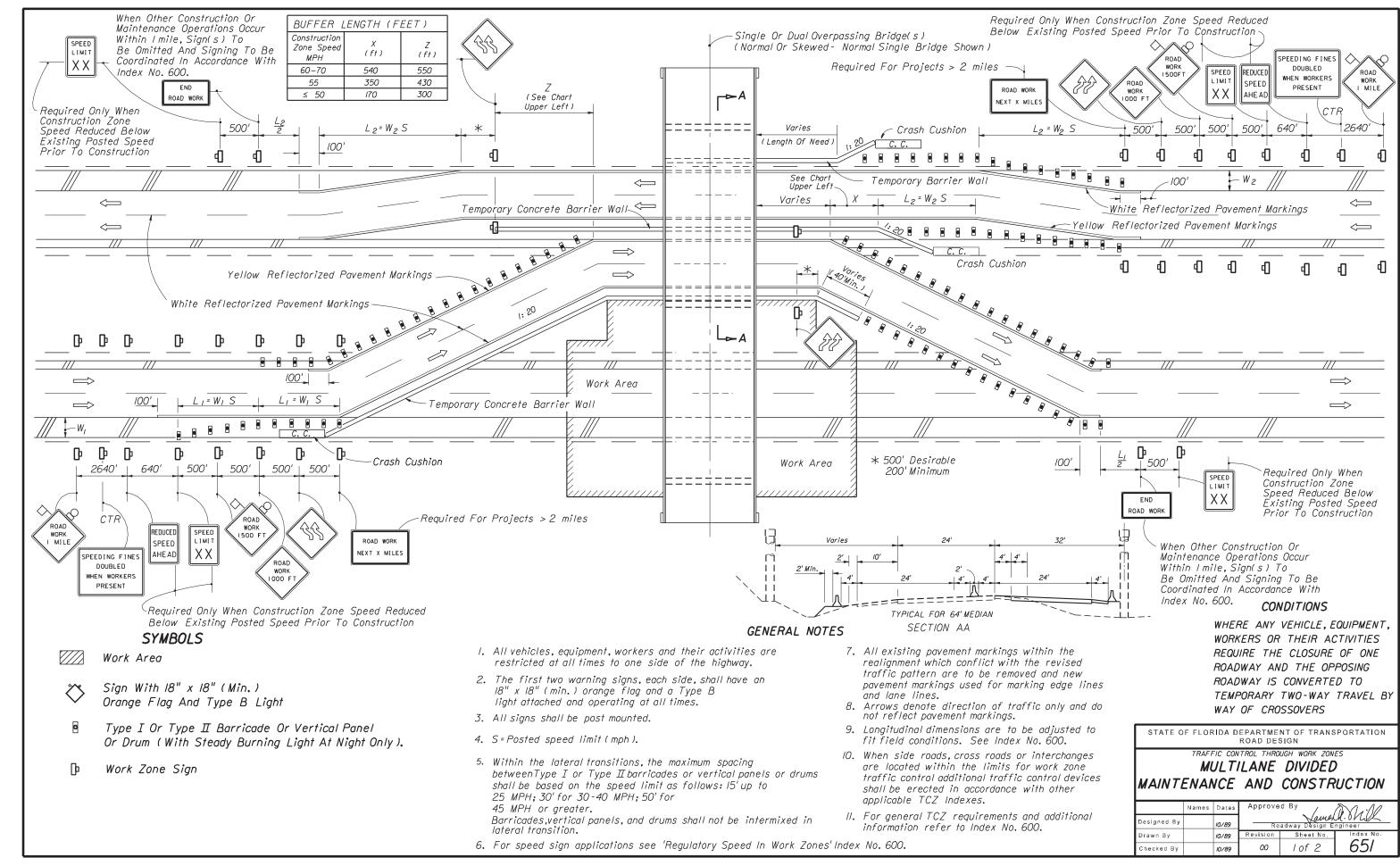
- I. All signing, pavement marking, barricades and warning lights necessary for maintenance of traffic shall conform to Index No. 600.
- 2. The first two warning signs shall have an 18" x 18"(min.) orange flag and a Type B light attached and operating at all times.
- 3. For speed sign applications see Index No. 600.
- 4. For lane width requirements see Index No. 600. When one-way one-lane operations are necessary, a minimum width of I2' shall be maintained and traffic controlled in accordance with Indexes Nos. 603, 604, 606, 607 or 608. Minimum width for the detour shoulders is 6'.
- 5. Method of attaching temporary guardrail to the diversion structure to be approved by the Engineer. Cost of temporary guardrail systems, including end anchorage assemblies, transitions and attachment to temporary structures, are to be included in the contract unit price for guardrail (temporary) LF.
- 6. Provisions approved by the Engineer shall be made for the removal of storm water from the roadway(s) during construction.
- 7. Temporary crash cushions shall be in accordance with Index No. 435 unless other device called for in the plans.
- 8. Arrows denote direction of traffic only and do not reflect pavement markings.
- 9. Longitudinal dimensions are to be adjusted to fit field conditions. See Index No. 600.
- 10. Where the temporary structure is not required the diversion may be constructed in accordance with Index No. 609, unless otherwise stipulated in the plans.
- II. For reflective raised pavement marker application see Index No. 600 and Index No. 17352.
- 12. For general TCZ requirements and additional information refer to Index No. 600.

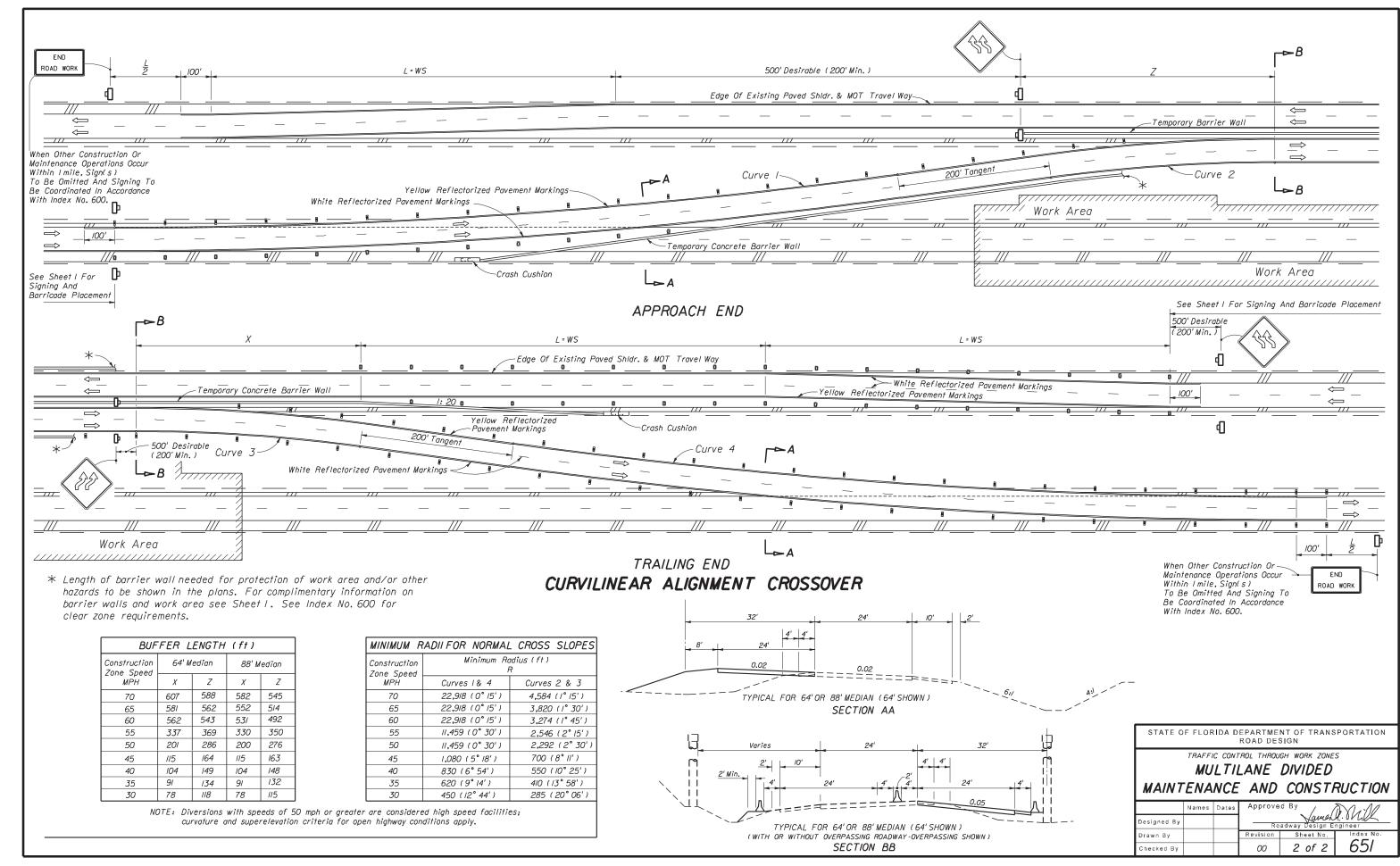
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

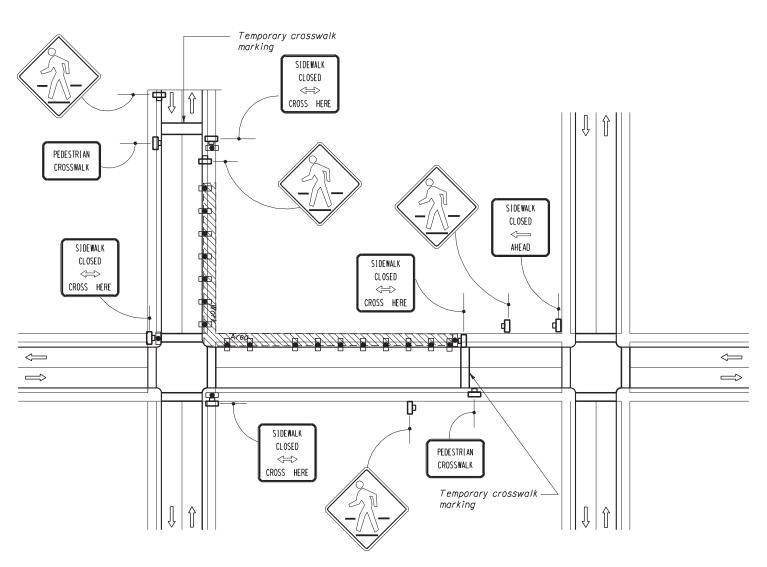
TRAFFIC CONTROL THROUGH WORK ZONES

TWO-LANE, TWO-WAY • RURAL STRUCTURE REPLACEMENT

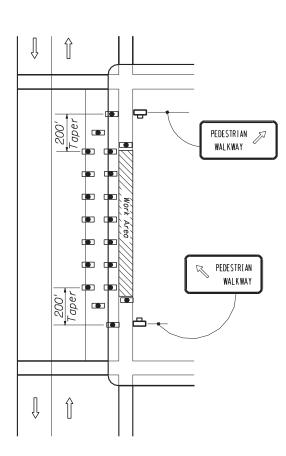
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Designed By		08/79	Ro	adway Design E	
Drawn By		08/87	Revision	Sheet No.	Index No.
Checked By		08/79	00	2 of 2	650







GENERAL NOTES



MID-BLOCK SIDEWALK CLOSURE
WITH TEMPORARY WALKWAY

TYPICAL APPLICATIONS

Sidewalk Repair Pavement Widening Utility Work

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT WORKERS OR THEIR ACTIVITIES ENCROACH ON THE SIDEWALK FOR A PERIOD OF MORE THAN 60 MINUTES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES

PEDESTRIAN CONTROL FOR CLOSURE OF SIDEWALKS

	Names	Dates	Approve	. /	Mille
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rawn By		7/93	Revision	Sheet No.	Index No.
Checked By		7/93	00	1 of 1	660

CORNER SIDEWALK CLOSURE WITH TEMPORARY CROSSWALKS

SYMBOLS

/// Work Area

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 \bigcirc

- Type I Or Type II Barricade Or Vertical Panel Or Drum (With Steady Burning Light At Night Only). (Tubular Markers May Be Used During Daylight Only. Cones May Be Used -See Index 600).
- ₩ork Zone Sign

- Arrows denote direction of traffic only and do not reflect pavement markings.
- 2. Only the signs controlling pedestrian flows are shown. Other work zone signs will be needed to control traffic on the streets.
- 3. For spacing of traffic control devices and general TCZ requirements refer to Index No. 600. Maximum spacing between barricades, vertical panels, drums or tubular markers shall not be greater than 25'.
- 4. Street lighting should be considered.
- 5. For nightime closures use Type A flashing warning lights on barricades supporting signs and closing sidewalks. Use Type C steady-burn lights on channelizing devices seperating the work area from vehicular traffic.
- 6. Pedestrian traffic signal display controlling closed crosswalks shall be covered or deactivated.

- 7. Temporary walkways shall be a minimum of 4' wide and kept free of any obstructions and hazards such as holes, debris, mud, construction equipment, stored materials and etc. (For details see Index 600)
- 8. Post Mounted Signs located near or adjacent to a sidewalk shall have a 7' minimum clearance from the bottom of sign to the sidewalk.

SIDEWALK CLOSED

<⇒> CROSS HERE

SIDEWALK CLOSED

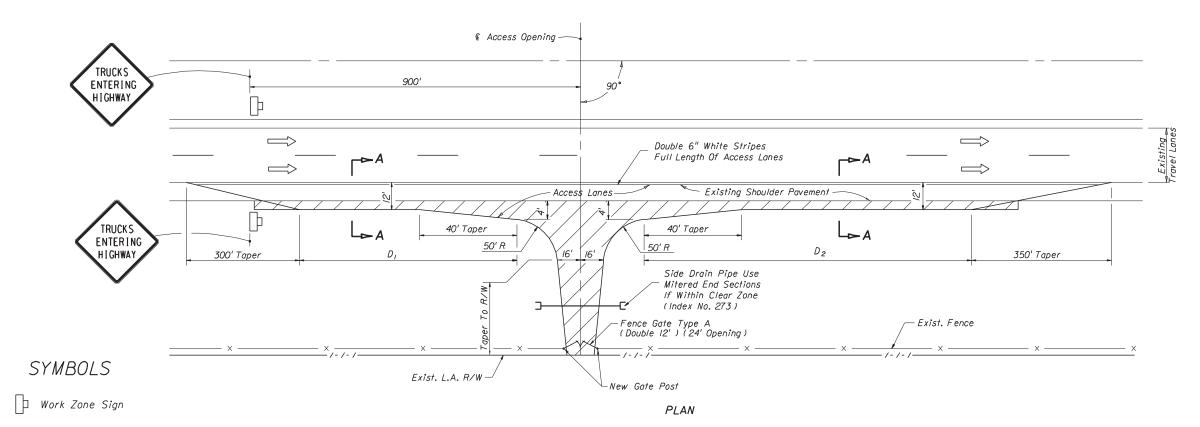
SIDEWALK

CLOSED

SIDEWALK CLOSED

CROSS HERE

- 9. When construction activities involve sidewalks on both sides of the street, efforts should be made to stage the construction so that both sidewalks are not out of service at the same time.
- 10. In the event that sidewalks on both sides of the street are closed, then pedestrians shall be guided around the construction zone.

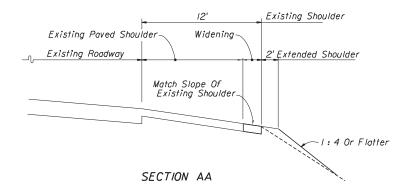


LENGTH OF ACC	CESS LANES	S (Ft)
Grade	D_I	D ₂
2% or less	590	1540
3 to 4% Upgrade	530	23/0

710

925

3 to 4% Downgrade



GENERAL NOTES

- I. Access openings across limited access right of way and use of this Index are prohibited unless specifically permitted in the Contract Plans or Special Provisions. When permitted in the Contract Plans or Special Provisions and prior to construction of any opening, the Contractor must submit, in writing, a request identifying specific locations for approval by the Engineer.
- 2. No more than two (2) access openings will be allowed on each project.
- 3. Access openings shall be located only in areas having adequate sight distance and shall not be located within 1.5 miles of interchanges nor within 2000 ft. of acceleration-deceleration lanes at rest areas, other access openings or other highway service areas.
- 4. Access openings shall not be constructed directly opposite temporary median crossovers nor within 2000 ft. of temporary median crossovers.
- Access openings shall be within the project limits and shall not be used for transporting materials to or from any other project. The acceleration-deceleration surfaces shall be paved. RAP material is acceptable for for driveway surfacing.
- 6. Any Motorist Aid Call Boxes affected by the temporary access openings shall be relocated outside the limits of access lanes and remain in use during construction. Upon removal of the access lanes, call boxes shall be returned to their previous location. Temporary relocation and restoration of call boxes shall be at the contractors expense.
- 7. Access openings in the limited access fence shall have gates which are to be locked during non-work hours or periods when the access is not in active use.
- 8. The contractor shall take all precautions necessary to insure against entrance by livestock or unauthorized persons or vehicles.
- 9. The contractor shall not vary from the plan detail without approval of the Engineer.
- 10. Gates shall be removed and access opening locations shall be restored to pre-construction condition immediately upon completion of activities utilizing the materials being transported through the openings whether or not the project is completed.
- II. Failure to comply with any provision of the access opening plan shall be cause for terminating use of all openings. Upon notification by the Engineer, the contractor shall cease hauling and begin restoration of affected areas. Under this condition expense of removal, restoration and of additional hauling distances shall be borne by the contractor.
- 12. No quardrail or barrier wall will be removed for access openings.
- I3. Construction and removal of the access and restoring the area to pre-construction condition shall be included in the cost of Maintenance Of Traffic, LS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC CONTROL THROUGH WORK ZONES
LIMITED ACCESS
RIGHT OF WAY
TEMPORARY OPENING

	Names	Dates	Approve	' /	Marson
Designed By			R	padway Design I	Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	I of I	665

NOTES

- I. Walls shall be constructed in accordance with Section 548 and the wall suppliers instructions.
- 2. Retaining Walls and all cast—in—place appurtenances, i.e., coping, traffic ralling barriers, sidewalk parapets, light pilasters, integral sign foundations, etc., shall be paid for at the contract unit price per square feet of retaining wall under, Retaining Wall System (Permanent), Retaining Wall System (Temporary). Payment shall be based on plan quantities.

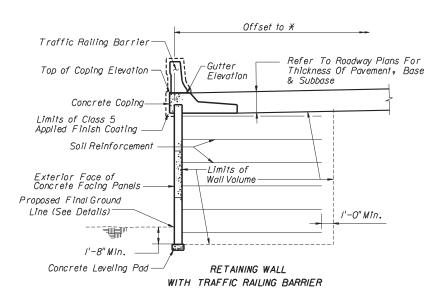
The related cost of installation of drainage structures (only structures affected by wall) shall be included in the unit cost for retaining wall, Pay Item Retaining Wall System (Permanent), or Retaining Wall System (Temporary).

- 3. All exposed surfaces of cast-in-place concrete shall receive a Class 5 Applied Finish Coating in accordance with Construction Specifications Section 400. Refer to Typical Wall Sections and the following notes for limits of applied finish:
 - a. The inside, backside and top of Traffic Railing Barriers and Pedestrian/Bicycle Railing Barriers.
 - b. Exposed surfaces of coping on top of retaining wall.
- 4. Other coatings, colors or textures shall be applied as required by the Contract Documents.
- 5. Piles within the wall volume shall be driven prior to construction of the retaining wall. The portion of the pile within the wall volume shall be wrapped with polyethylene sheeting in accordance with Section 459.
- 6. A structural extension of the connection of the wall panel to the soil reinforcement shall be used whenever necessary to avoid the cutting or excessive skewing (greater than 15 degrees) of the soil reinforcements around obstructions (i. e. piles, pipes, etc.).
- 7. For wall systems utilizing footings, the top of footing elevation is the same elevation as top of leveling pad.
- 8. Steps in leveling pads shall occur at panel interfaces. Panels shall not cantilever past the end of the leveling pad.
- 9. No cutting of soil reinforcement grids allowed unless shown on shop drawings and approved by the Engineer.

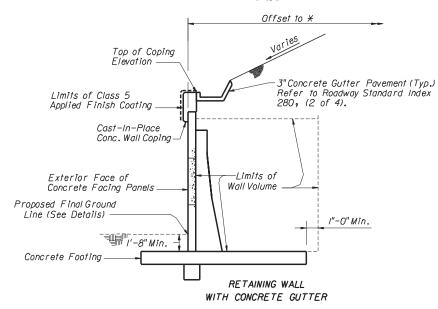
SHOP DRAWING REQUIREMENTS

The successful bidder shall submit the final design of the wall for review as shop drowings. The Shop Drowings shall include detailed design computations and all details, dimensions and quantities necessary to construct the wall. The design and fully detailed plans shall be prepared to Department standards current at time of bid and shall include, but not be limited to, presentation of required information as follows:

- I. Provide an elevation view of the wall indicating elevations at top of wall at begin and end wall stations, at all breaks in vertical alignment and at whole stations and 30 foot increments. Show elevations at top of leveling pad, bottom of footings, locations of all steps in leveling pad, panel designations, and length, size and designation of soil reinforcement in elevation view. Indicate location of the proposed final ground line.
- 2. Provide a plan view detailing the horizontal alignment and offsets from the horizontal control line(s) to the exterior face of the Wall.
- 3. Show in the plan and elevation all utilities, sign supports, light pole pilasters, drainage structures, drainage pipes, etc. that affect the walls. Locate on the plan all piles within the wall volume including those for future widening as shown on Foundation Layout drawings.
- 4. Provide general notes and design parameters on the shop drawings, Including design soil characteristics, minimum factors of safety, allowable material stresses and all other pertinent notes required for the construction of the walls. Provide the allowable and maximum actual bearing pressure for each wall height increment.
- 5. Show the limits of the wall volume.
- 6. Show all details of each concrete panel, slip joint and all other concrete elements incorporated in the wall, including reinforcing bar size and spacing, reinforcing bar bending details and details of all embedments.
- 7. Show all details of leveling pads and footings, including steps in leveling pads.
- 8. Show all details for construction of wall around obstructions. Show details for placement of soil reinforcement at acute corners and at interface with temporary walls.



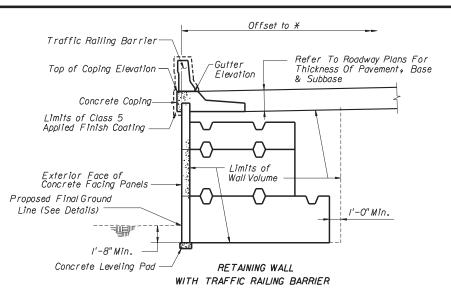
TYPICAL WALL SECTION - MSE SYSTEM (N.T.S.)



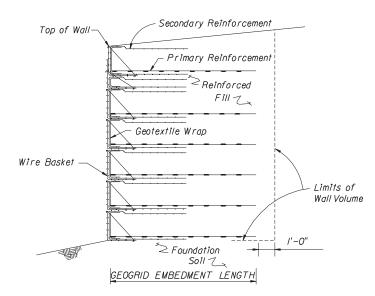
TYPICAL WALL SECTION - COUNTERFORT SYSTEM (N.T.S.)

* Insert control line designation i.e. \(\mathbb{E} \), \(\mathbb{E} \) etc.

- 9. Show all details addressing conflicts between soil reinforcement, concrete facing panels and embedments in the wall volume. Provide full details of connections of barriers, coping, sign supports, light pole pilasters, acute corners, etc.
- 10. Show all details where walls of different types intersect/influence one another.
- II. Provide fully detailed design calculations for each wall height increment utilized in the shop drawings. The submitted plans and design calculations shall be signed and sealed by a Professional Engineer registered in the State of



TYPICAL WALL SECTION - CONCRETE STEM SYSTEM (N.T.S.)



TEMPORARY WALL - TYPICAL CROSS-SECTION (N.T.S.)



See Roadway plans for complete drainage details.

DRAINAGE PIPE DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM GENERAL NOTES

	Names	Dates	Approve	d By	12/1
Designed By	RVR	11-98	St	ate Structures E	Oesign Engineer
Drawn By	JSP	11-98	Revision	Sheet No.	Index No.
Checked By	DEK	11-98	00	lof l	5000

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GENERAL NOTES

DESIGN CRITERIA

I. DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL WITHIN THE REINFORCED EARTH VOLUME, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED MATERIALS SHALL CONFORM TO THE CONTRACTING AGENCY'S TECHNICAL SPECIFICATIONS FOR RETAINED FARTH WALLS.

2. FACTORS OF SAFETY

OVERTURNING 2.0 INTERNAL PULLOUT 1.5 (ALLOW DEFORMATION 3/4") OVERALL STABILITY 1.5 SLIDING 1.5 BEARING 2.5

SOU REINFORCEMENT MESH 0.47 FV AT END OF DESIGN LIFE

3. SOIL CHARACTERISTICS ASSUMED FOR DESIGN:

SOU PARAMETERS:

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF 0, C AND & SHALL BE PROVIDED IN THE SHOP DRAWINGS.

- 4. THE MAXIMUM APPLIED BEARING PRESSURE AT THE FOUNDATION LEVEL IS AS SHOWN ON THE WALL ELEVATIONS FOR EACH DESIGN CASE. IT IS THE RESPONSIBILITY OF OTHERS TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 5 ANY LINSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED FARTH VOLUME, AS DETERMINED BY THE ENGINEER, SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL OR OTHERWISE STABILIZED AS DIRECTED BY THE ENGINEER.

REINFORCING ELEMENTS

6. REINFORCING MESH ELEMENTS SHALL BE SHOP FABRICATED FROM COLD DRAWN STEEL ROD CONFORMING TO THE MINIMUM REQUIREMENTS OF ASTM A-82 AND SHALL BE WELDED AT THE JUNCTIONS BETWEEN LONGITUDINAL AND TRANSVERSE WIRES IN ACCORDANCE WITH ASTM A-185. GALVANIZATION SHALL BE APPLIED AFTER MESH FABRICATION AND SHALL CONFORM TO THE MINIMUM REQUIREMENTS OF ASTM A-123.

LOOP EMBEDS SHALL BE FABRICATED FROM COLD DRAWN STEEL ROD COMFORMING TO ASTM A-510 OR ASTM A-82, LOOP EMBEDS SHALL BE WELDED IN ACCORDANCE WITH ASTM A-185, LOOP EMBEDS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM B-633.



6455 OLD PEACHTREE ROAD NORCROSS, GA 30071 Telephone: (770) 446-3000 Fax: (770) 242-7493

DESIGN:

7. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY OTHERS. ON THE BASIS OF THIS INFORMATION, THE WALL COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

WALL CONSTRUCTION

- 8A. (SQUARE PANELS) RETAINED EARTH WALLS IN CURVES WILL FORM A SERIES OF SHORT CHORDS OF 5.0'EACH TO MATCH DESIRED WALL ALIGNMENT.
- 8B. (HEX PANELS) RETAINED EARTH WALLS IN CURVES WILL FORM A SERIES OF SHORT CHORDS OF 4.33' EACH TO MATCH DESIRED WALL ALIGNMENT.
- 9. FOR LOCATION AND ALIGNMENT OF RETAINED EARTH WALLS, SEE RETAINING WALL CONTROL PLANS
- 10. IF MANHOLES AND DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN ON WALL ELEVATIONS.
- II. IF PILES ARE LOCATED WITHIN REINFORCED SOIL VOLUME. THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE REINFORCED
 EARTH WALL UNLESS A METHOD TO PROTECT THE STRUCTURE
 WHICH IS ACCEPTABLE TO THE ENGINEER AND FOSTER GEOTECHNICAL COMPANY AND IS PROPOSED AND APPROVED IN WRITING
- 12. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH SECTION 548 TO A LEVEL OF 2"(+/-) ABOVE THE TIE MESH EMBEDDED IN THE PANELS. INSTALLATION OF REINFORCING MESH SHALL BE PERMITTED ONLY AFTER PLACEMENT AND COMPACTION OF THE BACKFILL MATERIAL HAS REACHED THE REQUIRED LEVEL.
- 13. WALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH SECTION 548.
- 14. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE LOCATION OF ANY GUARDRAIL POSTS BEHIND RETAINED EARTH PANELS. PRIOR TO PLACEMENT OF THE TOP LAYER OF REINFORCING MESH, INDIVIDUAL REINFORCING MESH MAY BE SKEWED TO AVOID THE POST LOCATIONS IF AUTHORIZED BY THE ENGINEER (NO CUTTING OF SOIL REINFORCEMENT GRIDS ALLOWED UNLESS SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER). ANY DAMAGE DONE TO THE REINFORCING MESH DUE TO THE INSTALLATION OF THE GUARDRAIL SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- 15. IF EXISTING OR FUTURE STRUCTURES, PIPES, FOUNDATIONS OR GUARDRAIL POSTS WHICH ARE WITHIN REINFORCED SOIL VOLUME INTERFERE WITH THE NORMAL PLACEMENT OF REINFORCING MESH AND SPECIFIC DIRECTION HAS NOT BEEN PROVIDED ON THE PLANS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO DETERMINE WHAT COURSE OF ACTION SHOULD BE TAKEN.
- 16. TOP PANELS BENEATH CAST-IN-PLACE COPING SHALL HAVE *4 BARS PROTRUDING FROM THEIR TOP EDGE.
- 17. FOR OTHER INFORMATION PERTAINING TO WALL CONSTRUCTION PLEASE REFER TO FOSTER GEOTECHNICAL CONSTRUCTION MANUAL.
- 18. THE CONTRACTOR IS RESPONSIBLE FOR GRADUALLY DEFLECTING UPPER REINFORCING MESH DOWNWARD TO AVOID CONFLICTS WITH PAVING AND SURGRADE PREPARATION THE CONTRACTOR'S ATTENTION IS DIRECTED ESPECIALLY TO SITUATIONS WHERE ROADWAY SUPER ELEVATION AND/OR SOIL MIXING ARE ANTICIPATED.

MATERIALS NOTES

19. NOMINAL MESH LENGTHS

THE REINFORCING MESH LENGTH SHOWN ON THE PLANS, MEASURED FROM BACK FACE OF PANEL ARE THE NOMINAL LENGTHS REQUIRED BY CALCULATION. THE ACTUAL FABRICATED MESH LENGTHS ARE OFTEN LONGER (UP TO 6") DUE TO MANUFACTURING TOLERANCES. THE REQUIRED HORIZONTAL LIMIT OF GRANULAR BACKFILL IS EQUAL TO THE NOMINAL MESH LENGTH. ADDITIONAL GRANULAR BACKFILL BEYOND THE NOMINAL MESH LENGTH IS NOT REQUIRED BY CALCULATION.

20. REINFORCED BACKFILL QUANTITY

THE REINFORCED BACKFILL QUANTITY INDICATED BY FOSTER GEOTECHNICAL IS CALCULATED BY MULTIPLYING THE NOMINAL MESH LENGTHS SHOWN ON THE PLANS BY THEIR TRIBUTARY WALL SURFACE AREA AND CONVERTING THE RESULT TO A NEATER CUBIC METER QUANTITY. THIS INFORMATION IS FURNISHED FOR THE CONTRACTOR'S INFORMATION ONLY AND IS NOT INTENDED TO PRESENT THE ACTUAL QUANTITIES REQUIRED TO COMPLETE THE WORK. THE CONTRACTOR MUST CALCULATE HIS OWN EXCAVATION AND BACKFILL QUANTITIES BASED UPON THE SPECIFIC CONDITIONS OF THE PROJECT.

21. PANEL FINISH

THE PRECAST PANELS FOR THIS PROJECT SHALL BE A PLAIN STEEL FORM FINISH UNLESS OTHERWISE SPECIFIED ON THE RETAINED EARTH CONTROL

22. NOTE TO CONTRACTORS

ONLY THE FOLLOWING MATERIALS ARE SUPPLIED BY FOSTER GEOTECHNICAL

- PRECAST PANELS
- IOOP EMBED
- HDPE BEARING PAD (NOMINAL 4.0 MELT / .950 DENSITY)
- NON-WOVEN FILTER CLOTH AND ADHESIVE (FOR PANEL JOINTS ONLY) (WEBTECH-TERRATEX NO. 4 OR EQUAL)

ANY OTHER MATERIALS CALLED FOR IN THE CONTRACT PLANS OR SPECIFICATIONS ARE TO BE SUPPLIED BY THE CONTRACTOR. ANY JOINT MATERIALS SHOWN AT THE INTERFACE OF PRECAST PANELS AND CAST-IN-PLACE CONCRETE STRUCTURES ARE TO BE SUPPLIED BY THE ERECTION CONTRACTOR. ALL SANDBLASTING, PAINTING, SEALERS OR OTHER SPECIAL APPLIED COATINGS ARE ALSO SUPPLIED / INSTALLED BY THE CONTRACTOR IN THE FIELD FOLLOWING PANEL ERECTION.

23. FOSTER GEOTECHNICAL SUPPLIES PRECAST CONCRETE FACING PANELS AND ACCESSORIES TO BE USED IN CONJUNCTION WITH OTHER MATERIALS IN THE CONSTRUCTION OF RETAINED EARTH WALLS DETAILED HEREIN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL FURNISHED BY FOSTER GEOTECHNICAL IS INTENDED TO PROVIDE A GENERAL EXPLANATION OF THE SYSTEM. IT IS THE CONTRACTOR'S OBLIGATION TO DEVISE AND EXECUTE A PROJECT SPECIFIC ERECTION SEQUENCE. PANEL UNLOADING, HANDLING AND BRACING SYSTEM. AND FALL PROTECTION SYSTEM. THE BRACING SYSTEM SHOWN IN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL IS GENERAL IN NATURE AND DOES NOT ACCOUNT FOR PROJECT SPECIFIC CRITERIA COMPLIANCE WITH THE GUIDELINES IN THIS MANUAL DOES NOT RELIEVE THE CONTRACTOR OF ITS RESPONSIBILITY TO ADHERE TO THE PROJECT PLANS. SPECIFICATIONS AND CONTRACT DOCUMENTS OR COMPLIANCE WITH ALL FALL PROTECTION, SAFETY, LAWS, STANDARDS AND PROCEDURES AT THE JOBSITE. CONTRACTORS SHOULD TAKE SPECIAL PRECAUTIONS TO PREVENT THE PANELS FROM SHIFTING OR FALLING DURING THE ERECTION PROCESS.

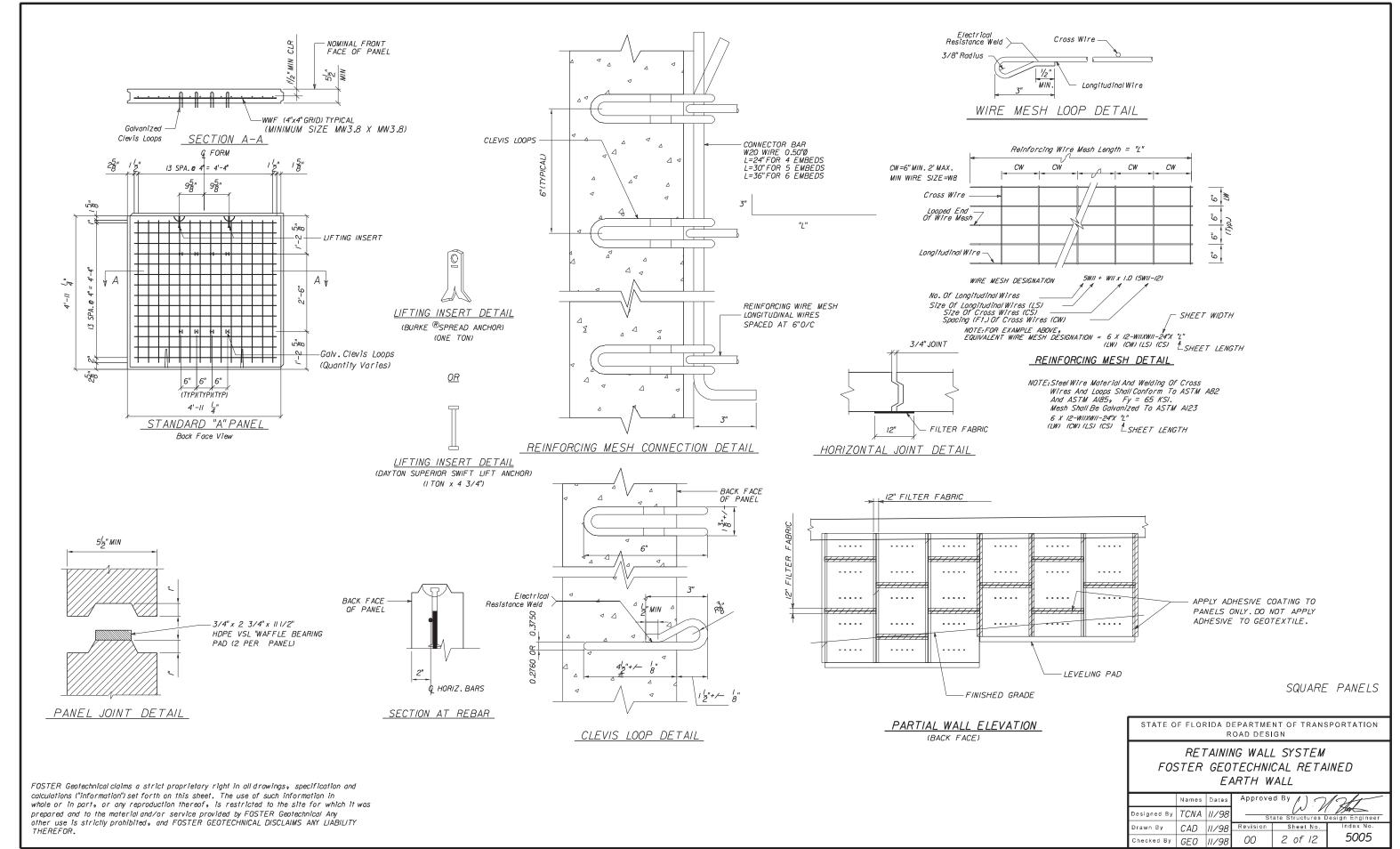
SQUARE / HEX PANELS

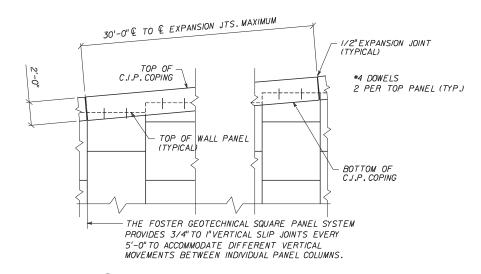
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

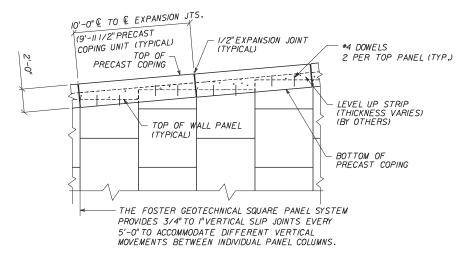
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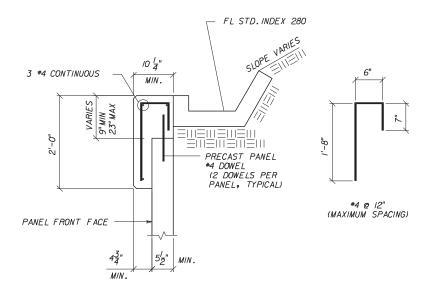
THIS SYSTEM SHALL BE USED IN MODERATELY OR SLIGHTLY AGRESSIVE ENVIRONMENTS ONLY.





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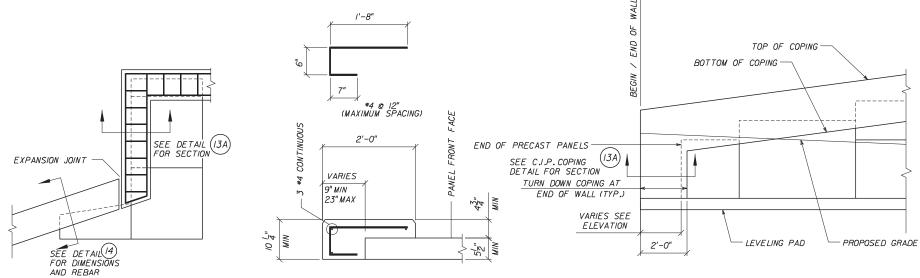


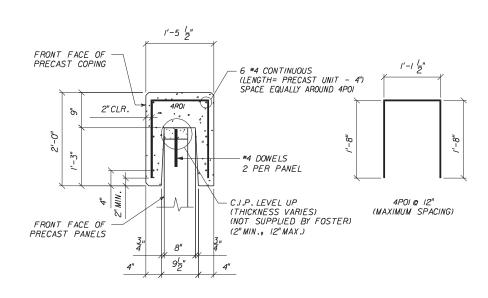


PARTIAL ELEVATION C.I.P. COPING
(SQUARE PANELS SHOWN, HEX PANELS SIMILAR)

PARTIAL ELEVATION PRECAST COPING
(SQUARE PANELS SHOWN, HEX PANELS SIMILAR)

14 C.J.P. COPING W/ DITCH
(2" MIN. COVER TYP.)





VERTICAL COPING (C.J.P.) SECTION
(2" MIN. COVER TYP.)

(SQUARE PANELS SHOWN,
HEX PANELS SIMILAR)

20 TYPE H_ PRECAST COPING

(STANDARD PRECAST COPING)

SQUARE / HEX PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

Designed By TCNA II/98

Drawn By CAD II/98

Checked By GEO II/98

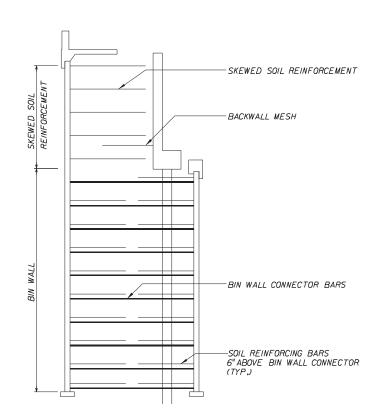
State Structures Design Engineer

Sheet No. Index No. 5005

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VERTICAL COPING (C.I.P.)

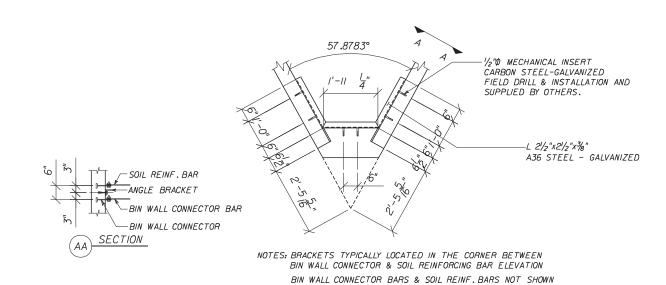
(SQUARE PANELS SHOWN, HEX PANELS SIMILAR)



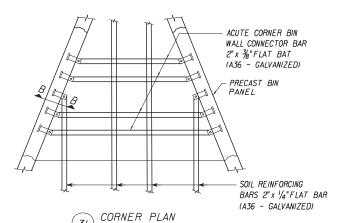
TYPICAL SECTION @ BIN WALL

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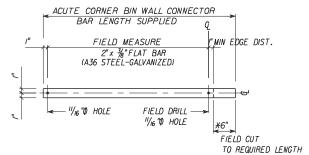
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ANGLE BRACKET DETAIL

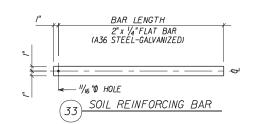


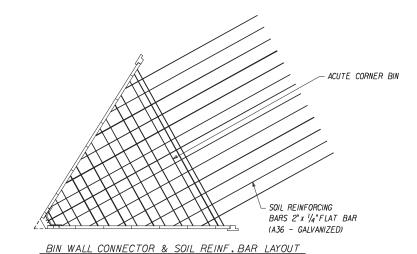
4" EMBED %" DOLT (A325 - GALVANIZED) SOIL REINFORCING BAR BIN WALL CONNECTOR BAR BIN WALL CONNECTOR (BB)

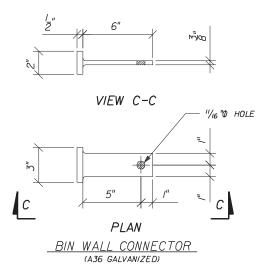


* EXPOSED STEEL ON FIELD MODIFIED END SHALL BE COATED WITH ZINC RICH PAINT

(32) BIN WALL CONNECTOR BAR







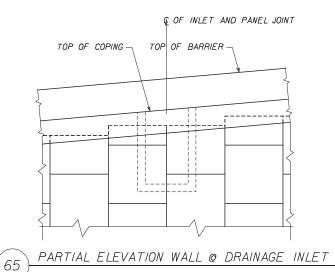
SQUARE / HEX PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

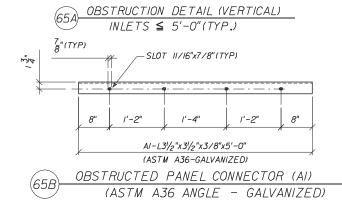
RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

	Names	Dates	Approve	d By /)	12/
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G OF INLET PLACE PANEL MESH IN OUTSIDE CLEVISES TO AVOID INLET. - INLET/MANHOLE SHALL BE CENTERED ON € OF THE CLOSEST "OBSTRUCTED **—** PANEL" JOINT. SEE WALL ELEVATIONS FOR INLET INVERT AND STATION OFFSET. Θ "OBSTRUCTED PANEL" SHALL HAVE L3½"x3½"x3%"x5'-0" ——— (ASTM A36-GALVANIZED) EIGHT(8) CLEVISES PER ROW. Θ * ONE "AI" PER OBSTRUCTED CLEVIS ROW



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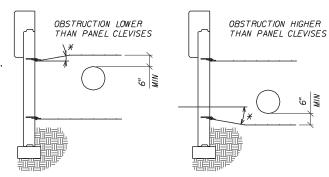
VERTICAL OBSTRUCTION NOTES

OBSTRUCTION SHALL BE CONSTRUCTED BEFORE WALL INSTALLATION.

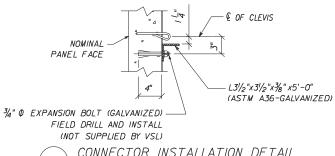
FIELD CUT AND SKEW MESH AROUND OBSTRUCTION AS REQUIRED. THESE AREAS WILL BE CLEARLY INDICATEDON THE RETAINED EARTH SHOP DRAWINGS AND APPROVED BY THE ENGINEER OF RECORD.

CUT MESH/DAMAGED GALV. SHALL BE COATED WITH ZINC RICH PAINT.

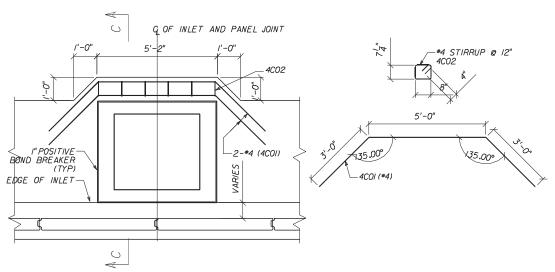
NO CUTTING OF SOIL REINFORCEMENT GRIDS ALLOWED UNLESS SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER



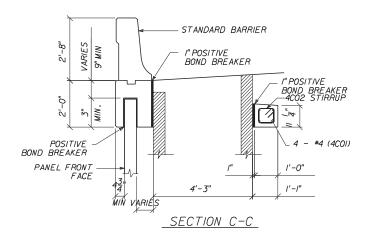
OBSTRUCTION (HORIZONTAL) * 15 DEGREES MAX BEND







PARTIAL PLAN - JUNCTION SLAB AROUND INLET (REBAR NOT SUPPLIED BY FOSTER GEOTECHNICAL)

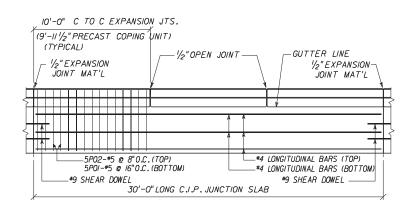


SQUARE / HEX PANELS

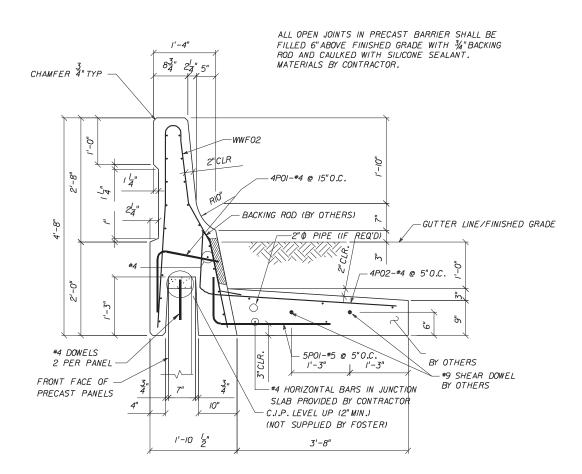
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

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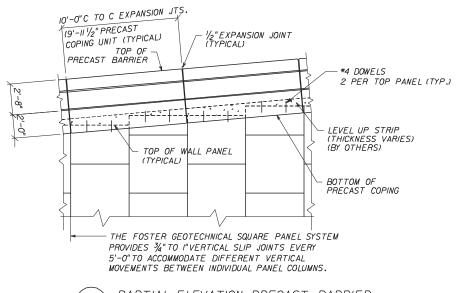


PLAN VIEW - PRECAST TRAFFIC BARRIER (HORIZONTAL BARS IN JUNCTION SLAB & #9 SHEAR DOWELS, NOT BY VSL)



TYPE HTB_ PRECAST BARRIER W/COPING & JUNCTION SLAB

U.S. PATENT NO. 4,494,892



PARTIAL ELEVATION PRECAST BARRIER (24B 4P0I-#4@I5"O.C. 0 2'-6" 6" 5P0/-#5 @ 5"0.C. WWF02 (4x6 - WI5.4xWI4.5) (LENGTH=PRECAST UNIT - 4") 4P02-#4@5"O.C. 4'-4"

SQUARE / HEX PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

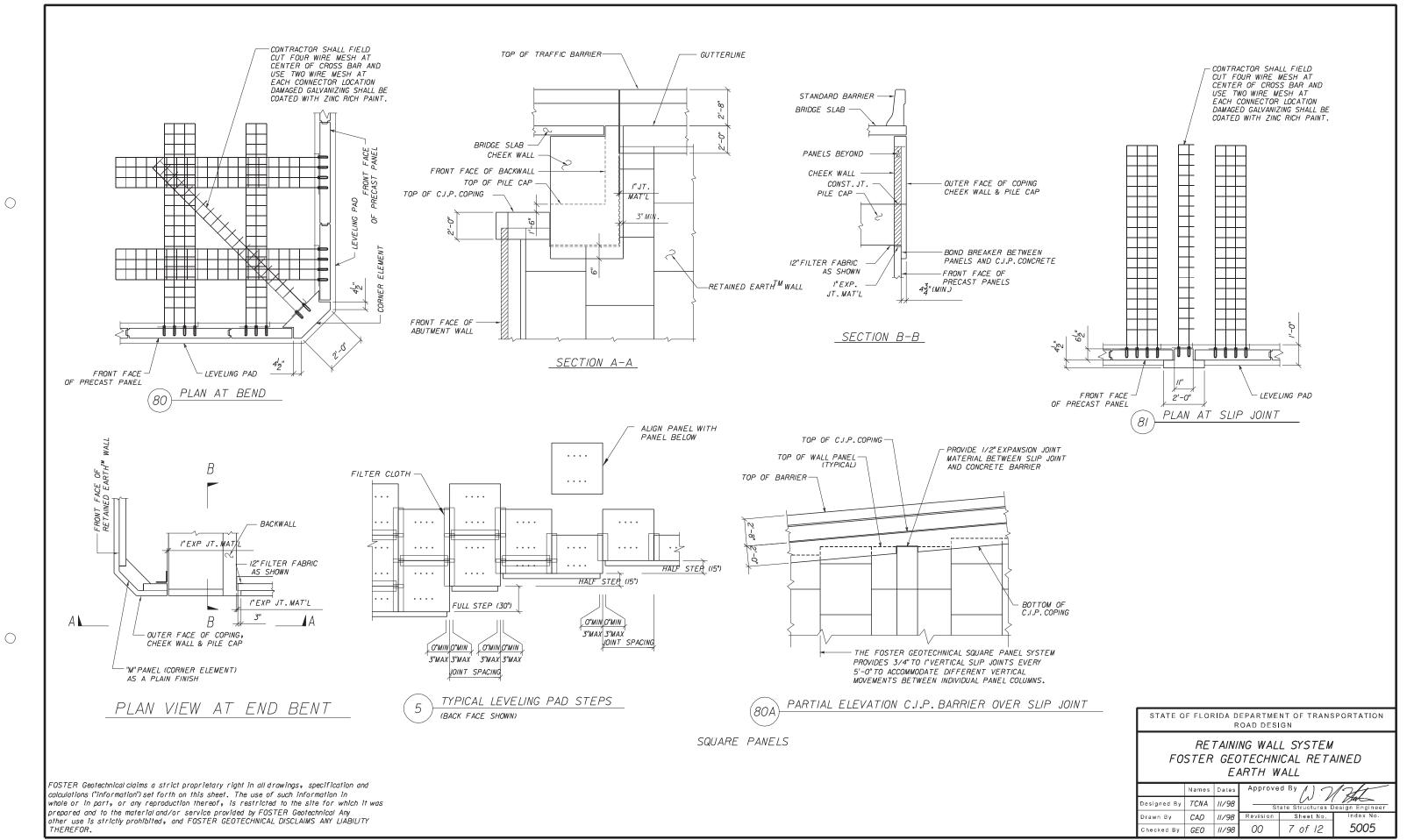
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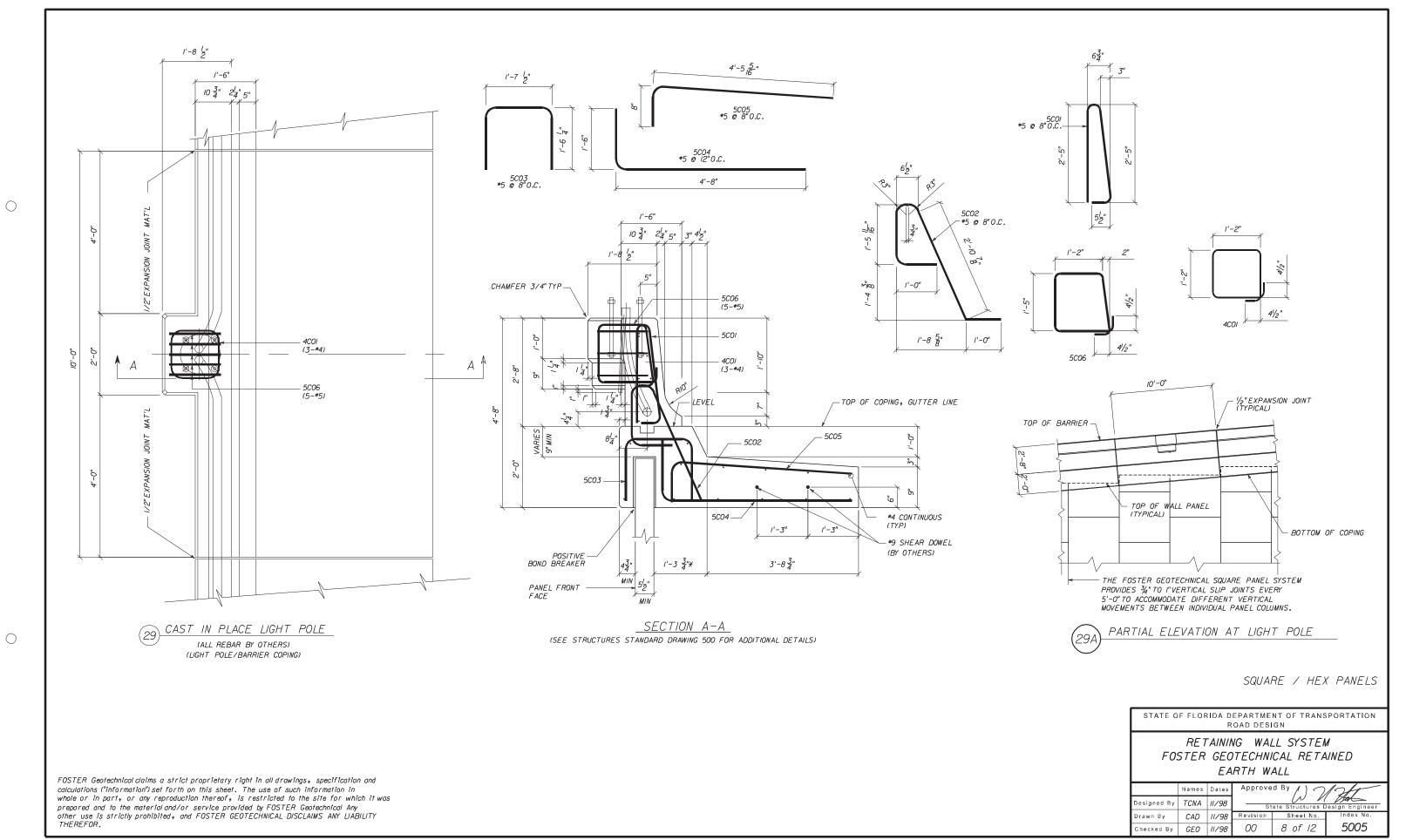
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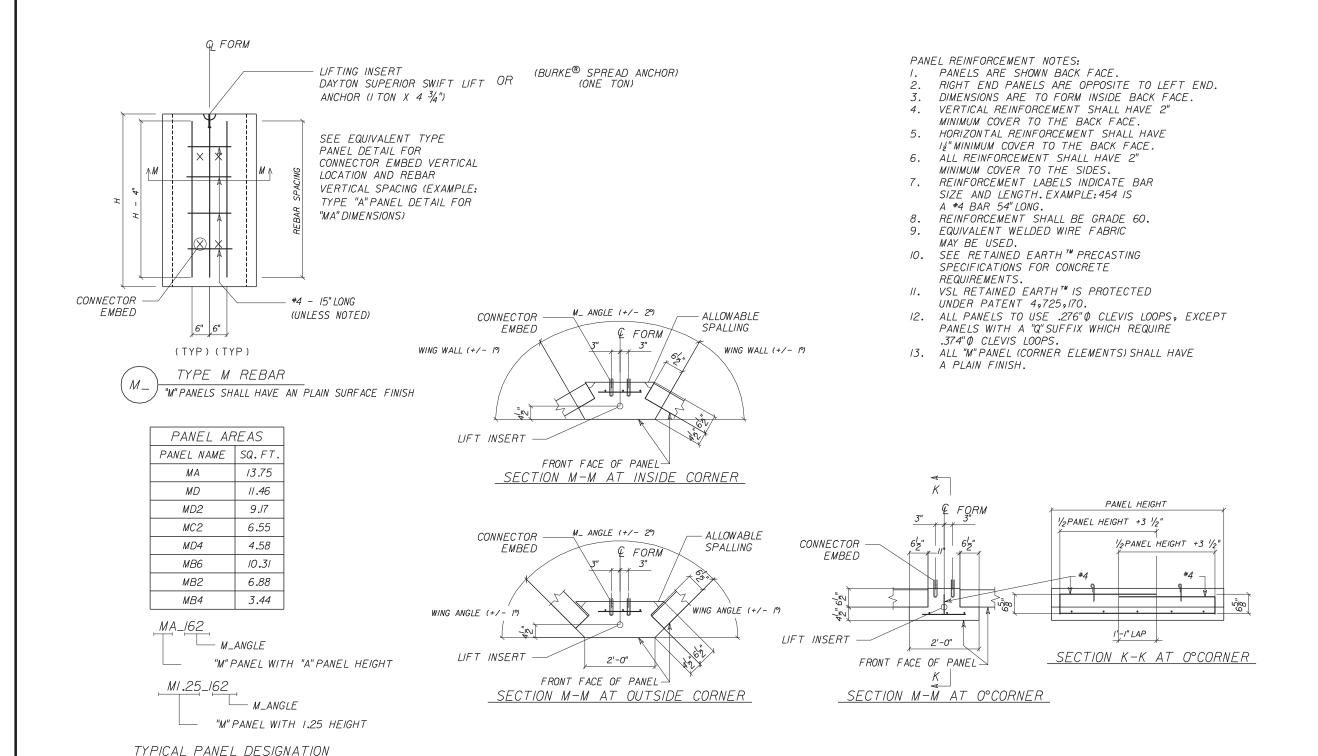
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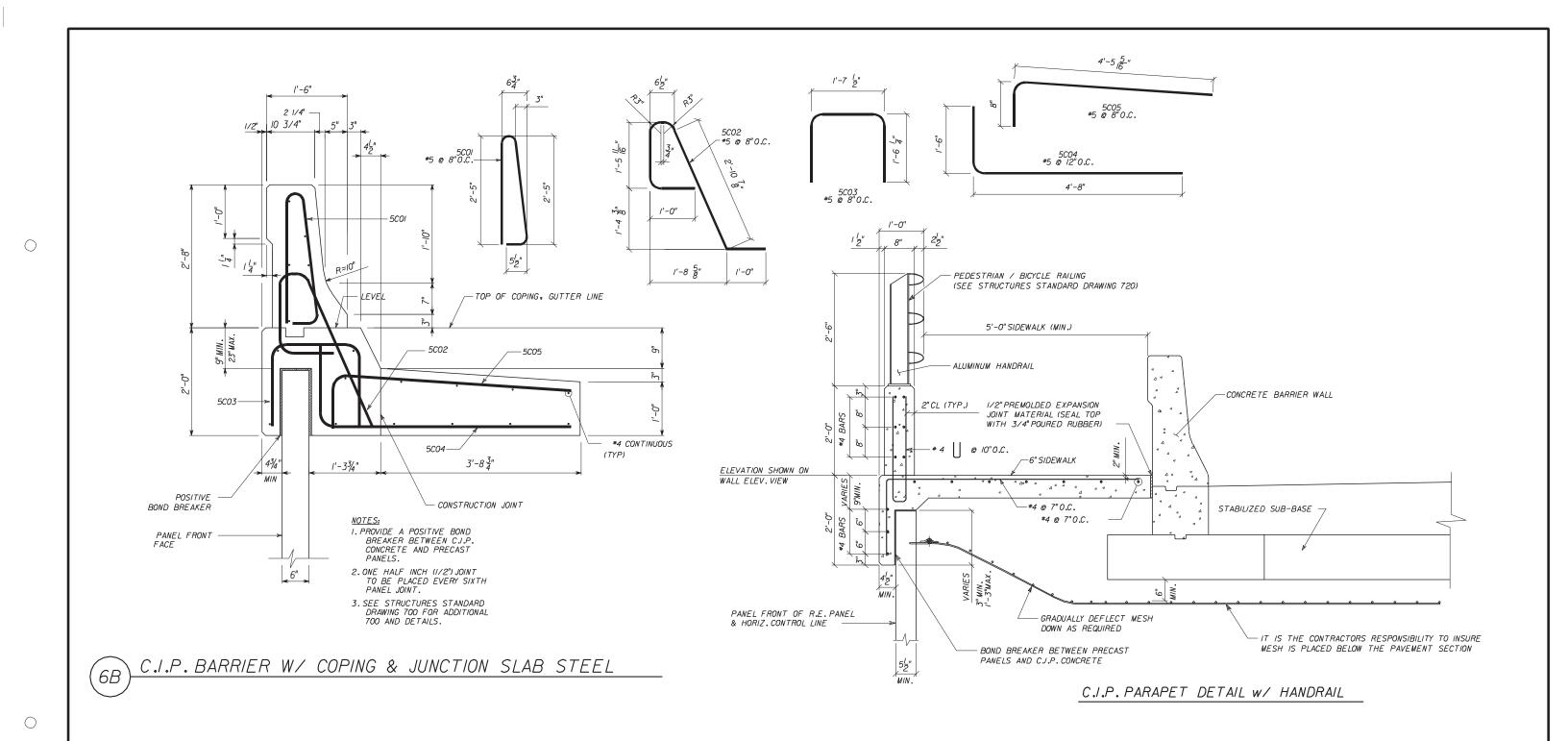
SQUARE / HEX PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

	Names	Dates	Approve	d By /)	12/1
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SQUARE / HEX PANELS

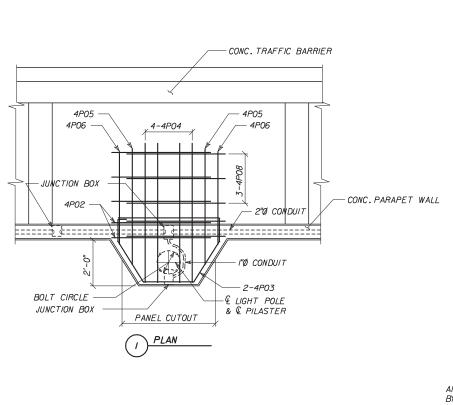
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

	Names	Dates	Approve	10 By / 1 5)	1211	
Designed By	TCNA	11/98	State Structures Design Engineer			
Drawn By	CAD	11/98	Revision	Sheet No.	Index No.	
Checked By	GE0	11/98	00	10 of 12	5005	

\$\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$\$ \$\$\$\$\$\$\$YTIME\$\$\$\$\$\$

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I. ADDITIONAL CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONSTRUCTION OF THE PILASTER SHALL MEET THE SAME REQUIREMENTS

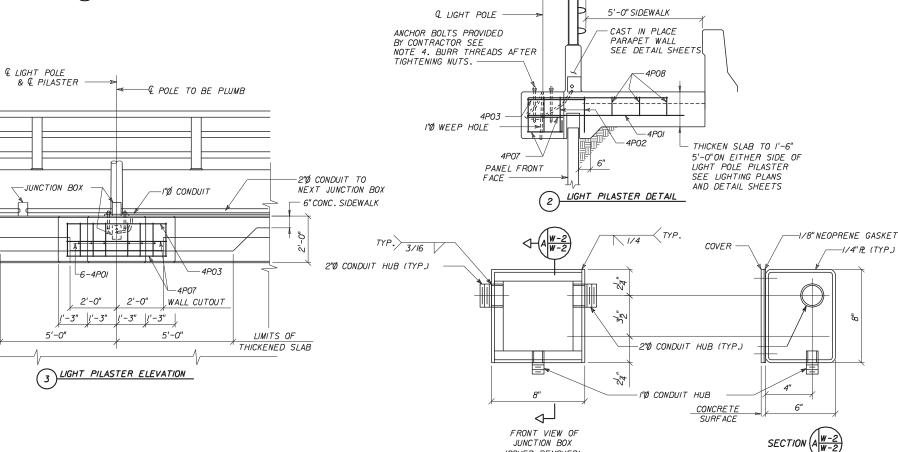
2. TOP OF PILASTER SHALL BE FINISHED TO A TRUE LEVEL AREA.

3. LIGHT POLE PILASTER IS DESIGNED TO RESIST WORKING LOADS (IN ANY DIRECTION) FROM THE LIGHT POLE APPLIED AT THE TOP OF THE PILASTER AS FOLLOWS:

AS THAT OF THE PARAPET WALL.

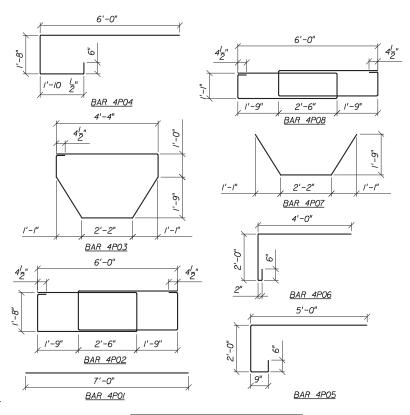
IF THE LIGHT POLE PROVIDED APPLIES LOADS THAT ARE IN EXCESS OF THOSE SHOWN ABOVE. THE CONTRACTOR SHALL REDESIGN THE PILASTER AND SUBMIT HIS DESIGN TO THE DEPARTMENT FOR REVIEW. THE CONTRACTOR'S REDESIGN SHALL BE PREPARED, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. AND QUALIFIED TO PERFORM THE WORK.

4. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ANCHOR BOLTS THAT EFFECTIVELY TRANSMIT THE LIGHT POLE LOADS TO THE PILASTER AND THAT FIT THE REINFORCING CAGE.CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA SHALL BE SUBMITTED BY THE CONTRACTOR TO THE DEPARTMENT FOR REVIEW AND APPROVAL SHOWING THAT THESE REQUIREMENTS HAVE BEEN MET PRIOR TO CONSTRUCTION.



(COVER REMOVED)

- 5. STEEL FOR JUNCTION BOXES SHALL CONFORM WITH ASTM-A36. THE BOXES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION. IN LIEU OF STEEL BOXES THE CONTRACTOR MAY SUBMIT FOR APPROVAL MOLDED P.V.C. BOXES (SCHEDULE 80).
- 6. ALL CONDUITS SHALL BE RIGID GALVANIZED STEEL OR SCHEDULE 80 P.V.C.
- 7. THE COST OF ANCHOR BOLTS SHALL BE INCLUDED IN THE BID PRICE FOR LIGHT POLES.
- 8. PAYMENT: THE COST OF ALL LABOR, CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONSTRUCTION OF THE PILASTERS AND ALL CONDUITS. EXPANSION COUPLINGS, JUNCTION BOXES AND MISCELLANEOUS HARDWARE REQUIRED FOR COMPLETION OF THE ELECTRICAL INSTALLATION WITHIN THE LIMITS SHOWN ON THIS SHEET SHALL BE INCLUDED IN THE CONTRACTOR'S BID PRICE FOR THE MSE WALLS.



BILL OF REINFORCING STEEL									
MARK	SIZE	NO. REQ'D	LENGTH						
4P0I	#4	6	7'-0"						
4P02	#4	2	24'-5"						
4P03	#4	1	13'-1"						
4P04	#4	4	10'-0 1/2"						
4P05	#4	2	8'-3"						
4P06	#4	2	6'-8"						
4P07	#4	2	6'-4"						
4P08	#4	3	22'-1"						

BAR BENDING DETAIL

SQUARE / HEX PANELS

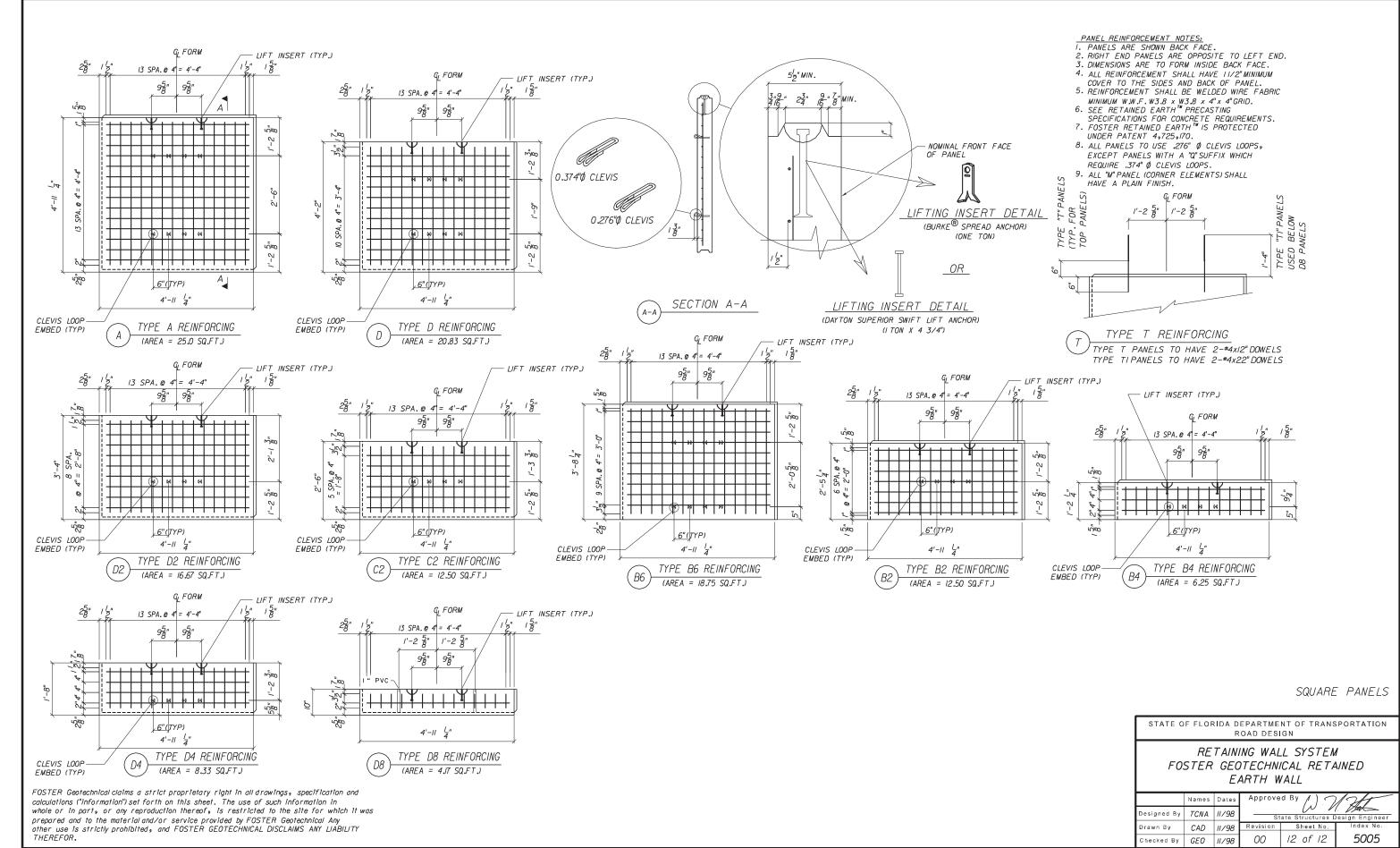
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL RETAINED EARTH WALL

	Names	Dates	Approve	d By	WHI -
Designed By	TCNA	11/98	St	ate Structures E	Design Engineer
Drawn By	CAD	11/98	Revision	Sheet No.	Index No.
Checked By	GE0	11/98	00	11 of 12	5005

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STANDARD DETAILS FOR 3"CONCRETE COVER

T-WALL® RETAINING WALL SYSTEM

DESIGNER



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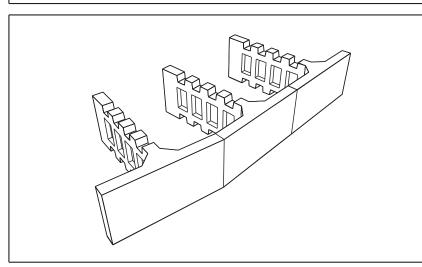
THE NEEL COMPANY

8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER

OLDCASTLE PRECAST, INC.

11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992



MISCELLANEOUS NOTES:

I. DESIGNER:
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VA 22152
PH: (703) 913-7858
FX: (703) 913-7859

2. PRECASTER:
OLDCASTLE PRECAST INC.
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: (904) 778-2990
FX: (904) 778-2992

3. MATERIALS SUPPLIED BY PRECASTER:

-PRECAST T-WALL UNITS

-PRECAST SHEAR KEYS

-HORIZONTAL JOINT MATERIAL

-VERTICAL JOINT MATERIAL AND ADHESIVE

-SHEAR KEY JOINT MATERIAL

DESIGN NOTES:

I. DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL WITHIN THE RETAINING WALL VOLUME, METHODS OF CONSTRUCTION, AND QUALITY OF PREF ABRICATED MATERIALS SHALL CONFORM TO SPEC SECTION 548 - RETAINING WALL SYSTEMS.

2. SOIL PARAMETERS:

-SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS
OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN
OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE
SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED
ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE
SITE. THE VALUE OF 0, C AND GAMMA SHALL BE PROVIDED
IN THE SHOP DRAWINGS

3. FACTORS OF SAFETY:
-OVERTURNING - 2.0
-SLIDING - 1.5
-INTERNAL PULLOUT - 1.5
-BEARING CAPACITY - 2.5
-OVERALL STABILITY - 1.5

4. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY THE OWNER. ON THE BASIS OF THIS INFORMATION, THE NEEL COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN, INCLUDING FOUNDATION AND SLOPE STABILITY, IS THE RESPONSIBILITY OF OTHERS.

5. PANELS WITH CANTILEVERED (EXTENDED) FACE SHALL ONLY BE USED TO AVOID OBSTRUCTIONS AS APPROVED ON THE SHOP DRAWINGS.

MATERIALS NOTES:

I. PRECAST CONCRETE:
-PRECAST T-WALL UNITS - PER SPEC
SECTION 548
-PRECAST SHEAR KEYS - PER SPEC
SECTION 548

2.C.I.P.CONCRETE:
-C.I.P.LEVELING PAD - PER SPEC
SECTION 548
-OTHER C.I.P.CONCRETE - PER SPEC
SECTION 548

3. REINFORCING STEEL:
-PER SPEC SECTION 548

4. JOINT MATERIAL:

-HORIZONTAL JOINT FILLER:

-JCZN 4" x 5"-0"

-PREFORMED EPDM

-DUROMETER: 80 - 90

-VERTICAL JOINT COVER:

-TENSAR DC4205 OR EQUAL

-JZ"WIDE x HEIGHT OF JOINT

-GEOCOMPOSITE MEETING REQUIREMENTS

OF SPEC SECTION 548

-SHEAR KEY WRAP:

-J/4" x 8" x 24"

-AVI ASTRO-FOAM AF-250

5. BACKFILL: -PER SPEC SECTION 548

CONSTRUCTION NOTES:

- I. ALL CONSTRUCTION PROCEDURES SHALL COMPLY WITH SPEC SECTION 548 AND THE "T-WALL CONSTRUCTION MANUAL" (PROVIDED BY THE NEEL COMPANY OR OLDCASTLE PRECAST, INC), IN THE EVENT OF A DISCREPANCY BETWEEN THE SPEC AND THE "T-WALL CONSTRUCTION MANUAL", THE SPEC SHALL CONTROL.
- 2. FOR LOCATION AND ALIGNMENT OF T-WALL STRUCTURE, SEE RETAINING WALL CONTROL PLANS.
- 3.T-WALL STRUCTURES ON CURVES SHALL BE BUILT IN CHORDS AS SHOWN IN THE T-WALL DESIGN DRAWINGS.
- 4. IF MANHOLES OR DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN IN THE T-WALL DESIGN DRAWINGS.
- 5. IF PILES ARE LOCATED WITHIN THE RETAINING WALL VOLUME, THEY SHALL BE DRIVEN BEFORE CONSTRUCTION OF THE T-WALL STRUCTURE.
- 6.T-WALL UNITS SHALL BE PLACED ONE ROW AT A TIME, AND BACKFILLED BEFORE PLACEMENT OF THE NEXT ROW.

 7.IF A STRUCTURE EXCEEDS 20' IN HEIGHT, THE FINISH GRADE AT THE FACE OF THE WALL SHALL BE PLACED AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS 20' IN HEIGHT.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR CONTROLLING STORM WATER DRAINAGE IN THE VICINITY OF THE WALL DURING CONSTRUCTION. STORMWATER RUNOFF SHALL BE COLLECTED AND DISCHARGED AWAY FROM THE WALL AND THE RETAINING WALL VOLUME.

THIS SYSTEM SHALL NOT BE USED FOR WALLS WITH ACUTE INTERIOR CORNERS IN SALT WATER ENVIRONMENTS.

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

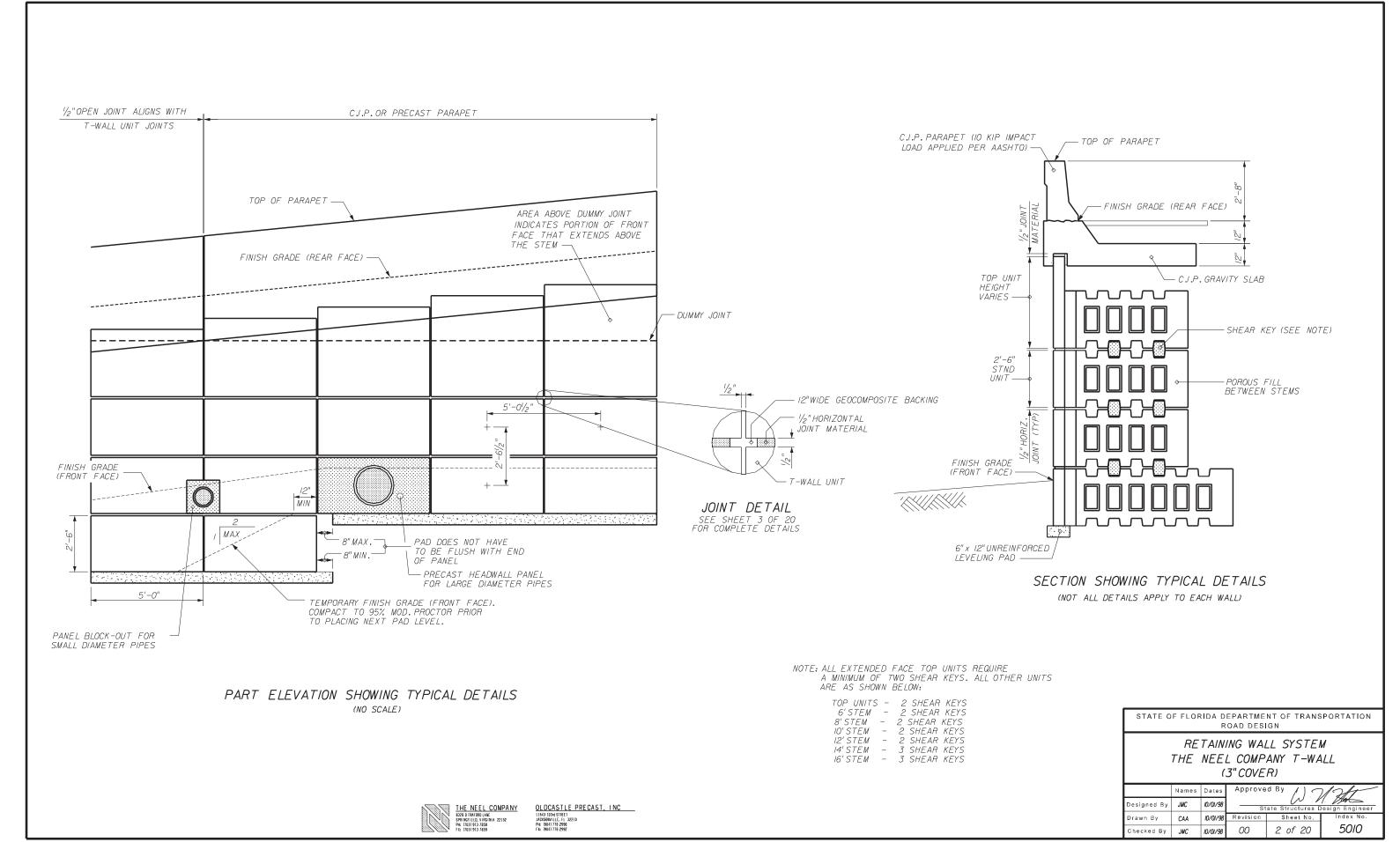
RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3"COVER)

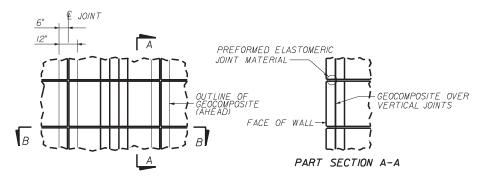
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signed By	JMC	10/01/98	State Structures Design Engineer			
awn By	CAA	10/01/98	Revision	Sheet No.	Index No.	
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THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD. VIRGINIA 22152
PH. 17031 913-7858

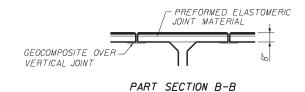
OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PPk: 19041 778-2990
FX: 19041 778-2992

\$\$\$\$\$\$DGN\$PECIFICATION\$\$\$\$\$\$\$





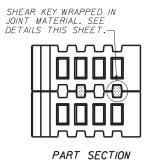
PART ELEVATION - REAR FACE

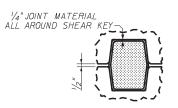


NOTES:

- 2. VERTICAL JOINT:
 //c"SPACE
 12" WIDE GEOCOMPOSITE BACKING, CENTERED
 ABOUT JOINT CENTERLINE.

JOINT MATERIAL DETAILS





SHEAR KEY / JOINT MATERIAL ARRANGEMENT

NOTES:

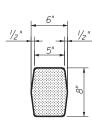
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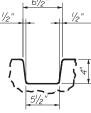
- 1. SHEAR KEY JOINT MATERIAL:

 MINIMUM OF ONE '/a" x 8" x 24" PIECE OF
 AVI ASTRO-FOAM AF-250 PER SHEAR KEY.

 2. JOINT MATERIAL MAY BE ADDED OR REMOVED
 TO AID IN SHIMMING AND ALIGNING, HOWEVER
 SHEAR KEY MUST FIT SNUG IN THE SHEAR KEY
 BLOCKOUT WHEN UNIT IS IN ITS FINAL POSITION.
- 3. MINIMUM OF 2 SHEAR KEYS REQUIRED PER UNIT. SEE NOTES ON SHEET 2 OF 20, 'TYPICAL DETAILS (1)'.



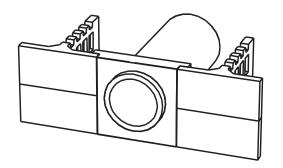


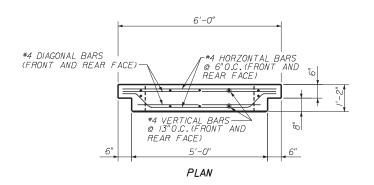


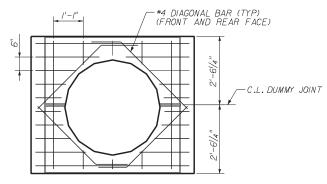
SHEAR KEY DIMENSIONS

SHEAR KEY BLOCKOUT DIM's

SHEAR KEY DETAILS







ELEVATION (FRONT FACE) PRECAST HEADWALL PANEL FOR LARGE DIAMETER PIPES

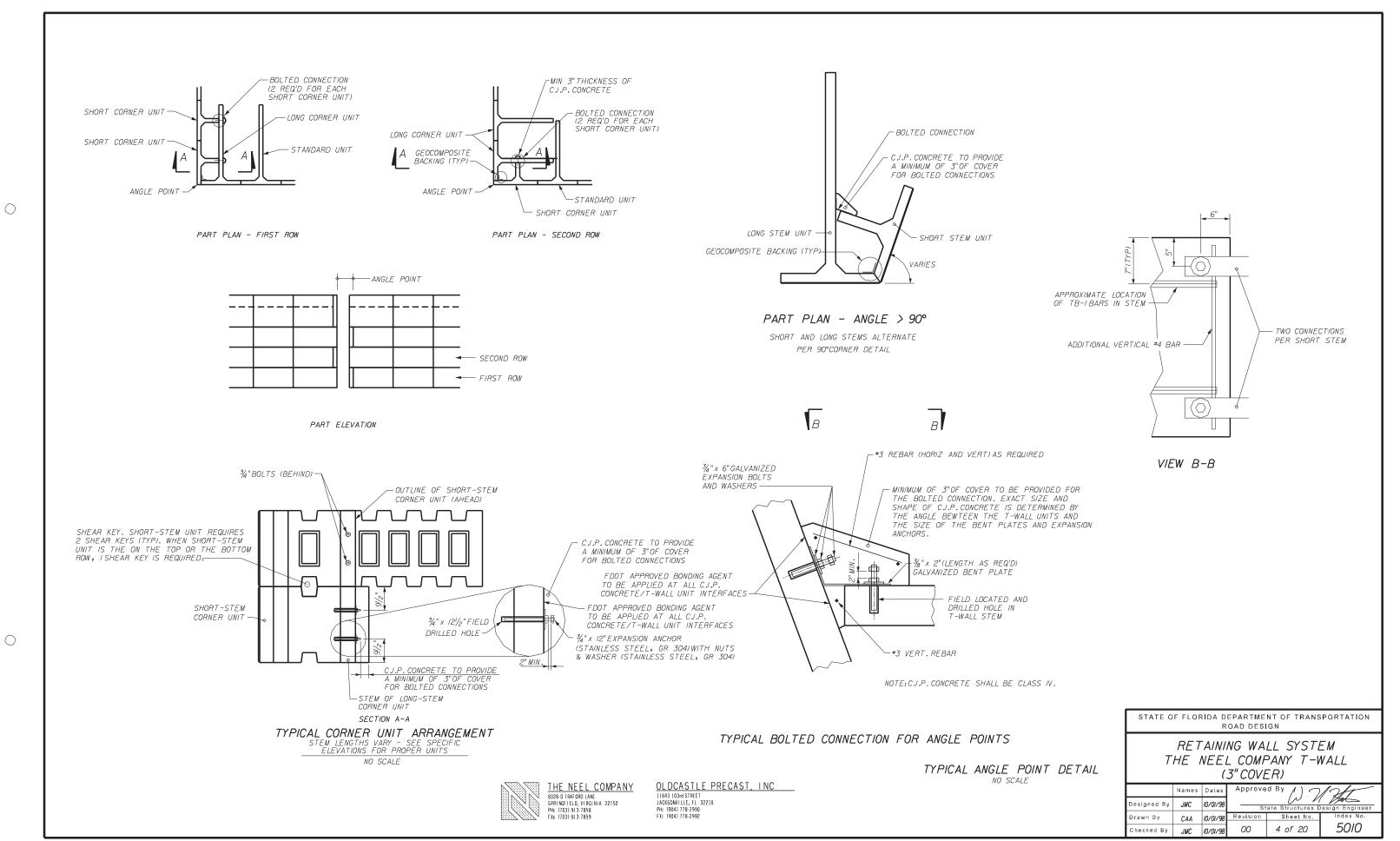
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

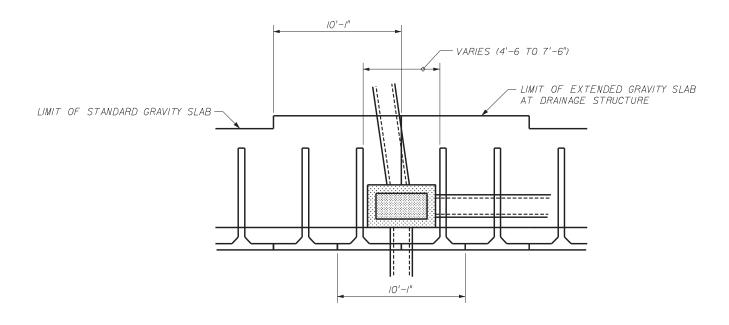
> RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (3" COVER)

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esigned By	JMC	10/01/98	State Structures Design Engineer			
rawn By	CAA	10/01/98	Revision	Sheet No.	Index No.	
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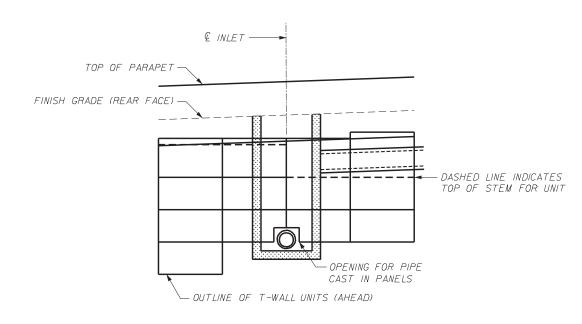
THE NEEL COMPANY

OLDCASTLE PRECAST, INC

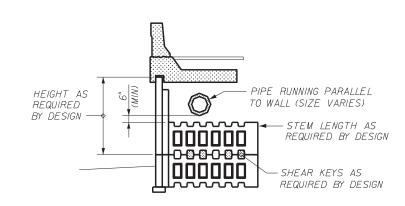




PART PLAN







SECTION
(SHOWING PIPE PARALLEL TO WALL)

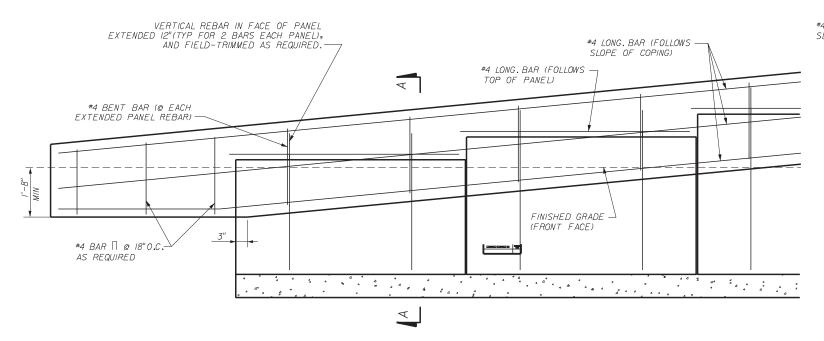


OLDCASTLE PRECAST, INC
11643 103rd \$1REET
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Pk: 19041778:2990
FX: 19041778:2992

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

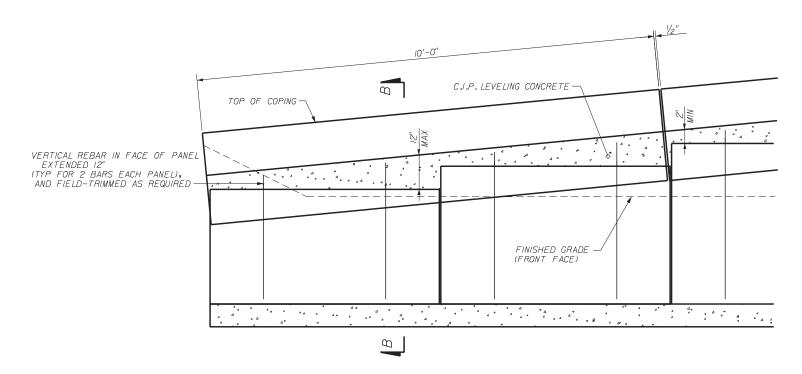
RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3" COVER)

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Designed By	JMC	10/01/98	State Structures Design Engineer			
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.	
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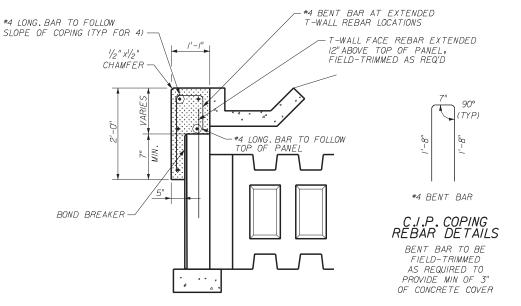


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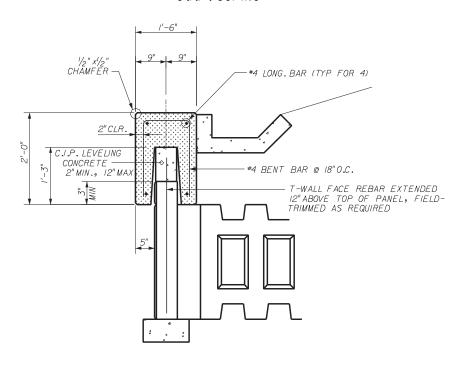
C.I.P. COPING TREATMENT AT BEGINNING/END OF WALLS



PRECAST COPING - PART ELEVATION



SECTION A-A C.J.P. COPING



SECTION B-B PRECAST COPING

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM

THE NEEL COMPANY T-WALL
(3"COVER)

ı		Names	Dates	Approve	d By /) 📿	12/	
	Designed By	JMC	10/01/98	State Structures Design Engineer			
ı	Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.	
	Checked By	JMC	10/01/98	00	6 of 20	5010	

THE NEEL COMPANY

8328-D TRAFORD LANE
SPRING: IELD, VIRGINIA 22152
PH: 17031 913-7859

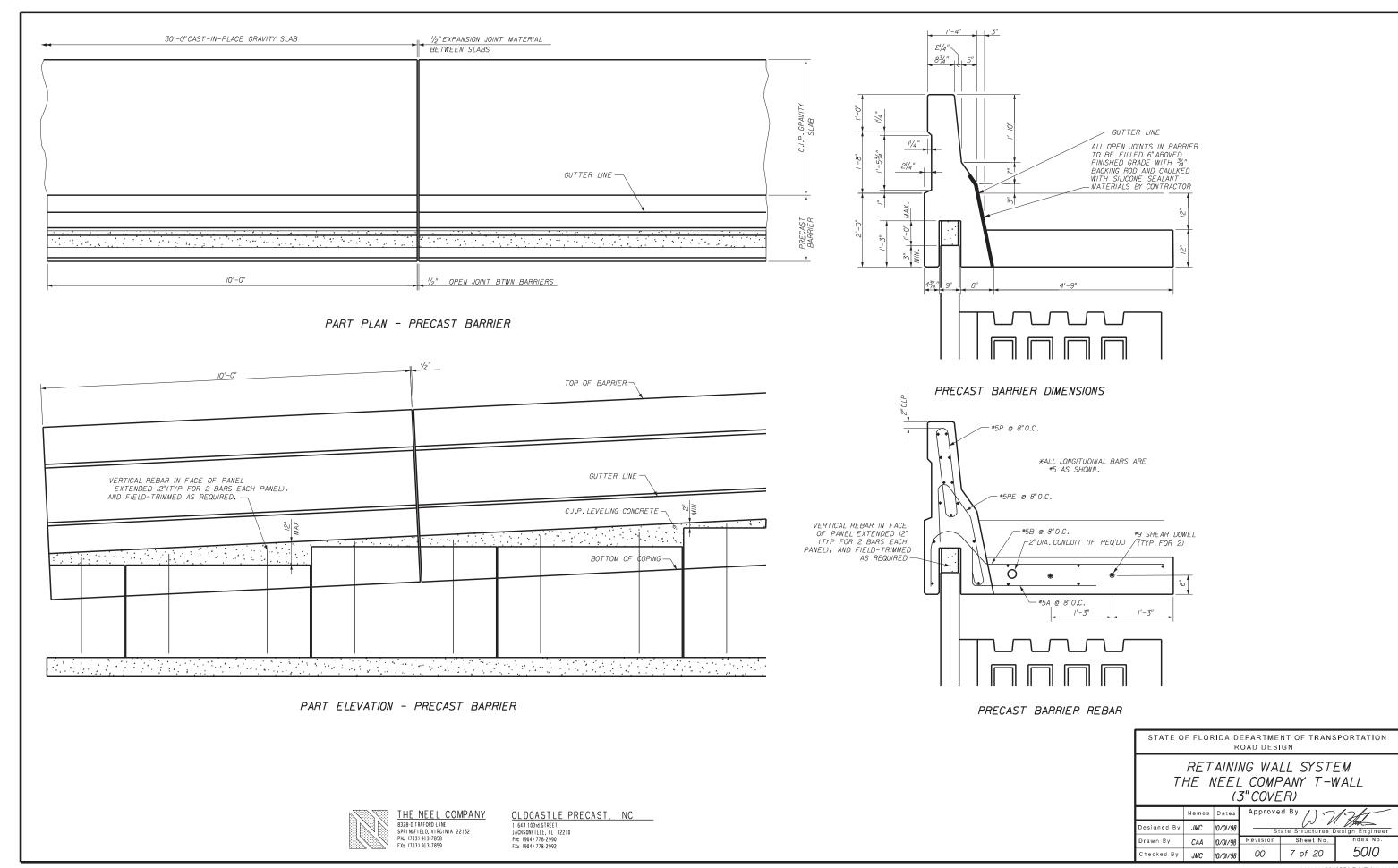
FX: 17031 913-7859

 EL COMPANY
 OLDCASTLE PRECAST, INC

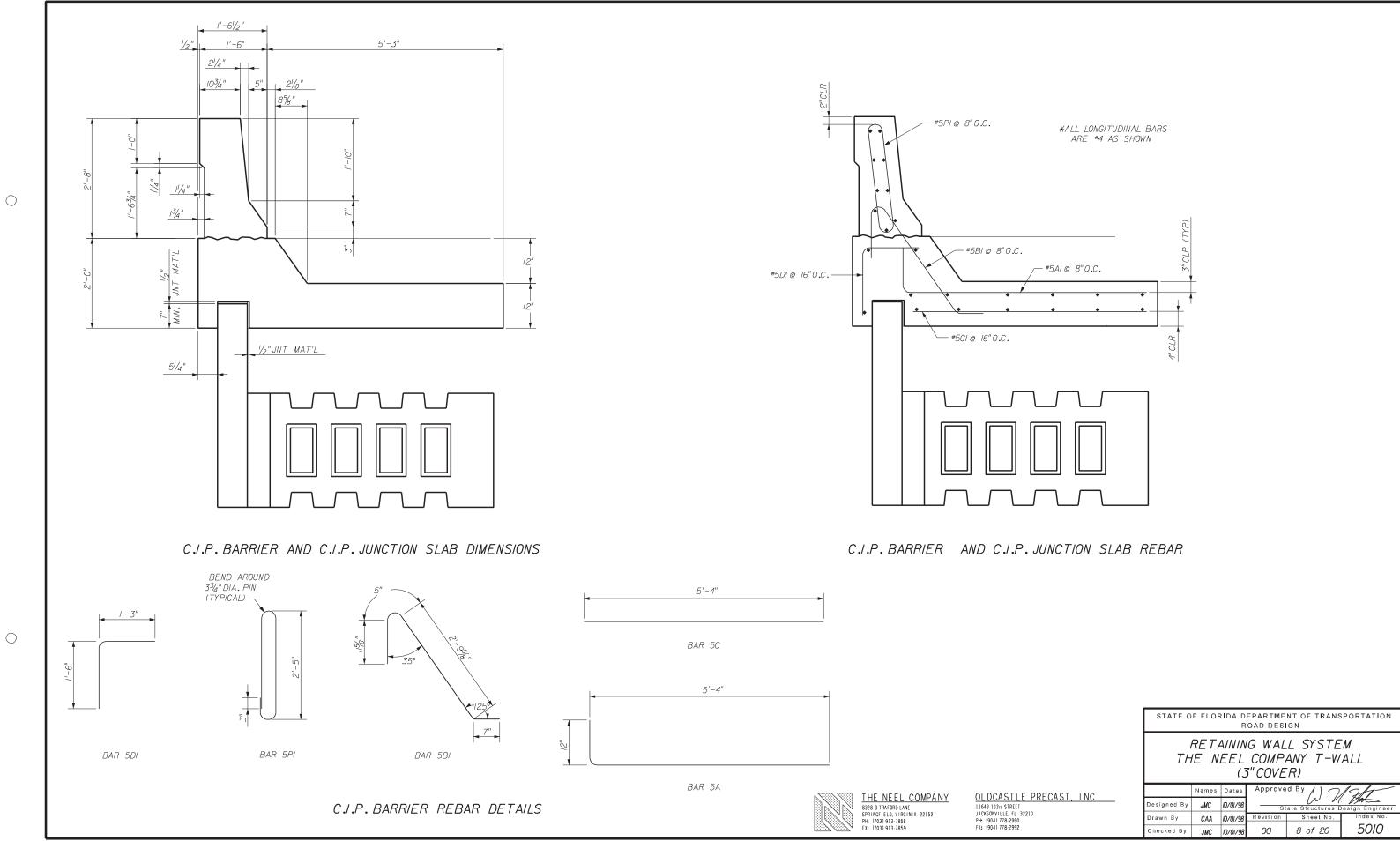
 DLANE
 11643 10394 STREET

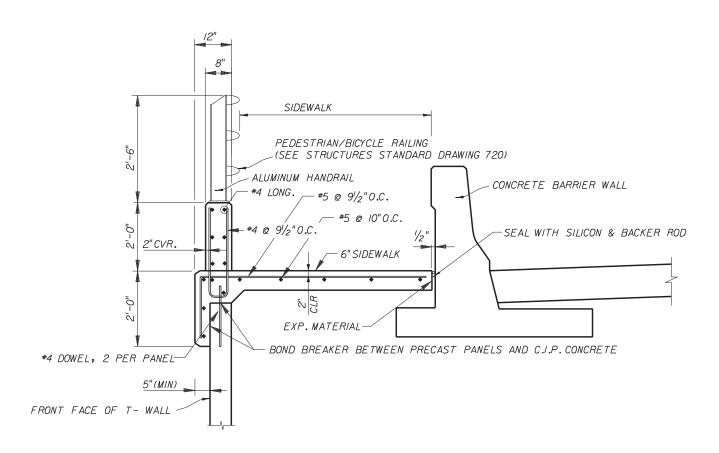
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 11643 10394 FEET 2310.0

11643 103rd STREET JACKSONVILLE, FL 32210 PH: {904} 778-2990 FX: {904} 778-2992



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C.I.P. PARAPET DETAIL W/ HANDRAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

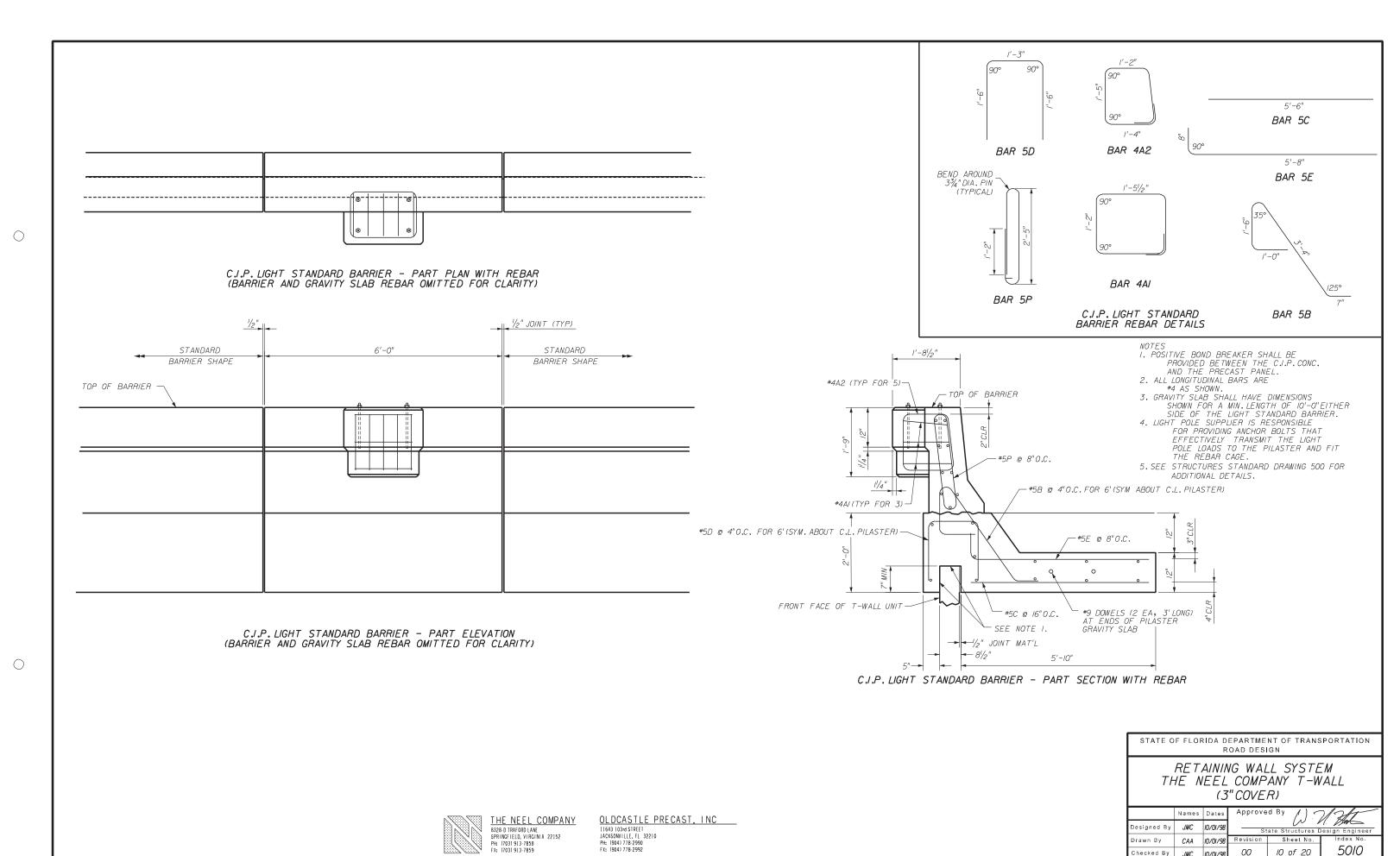
RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3"COVER)

THE NEEL COMPANY
8328-0 TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031 913-7858
Fx: (7031 913-7859

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OLDCASTLE PRECAST, INC
11643 103rd STREE1
JACKSOWILLE, FL 32210
Ptk 19941 778-2990
FX: 19041 778-2992

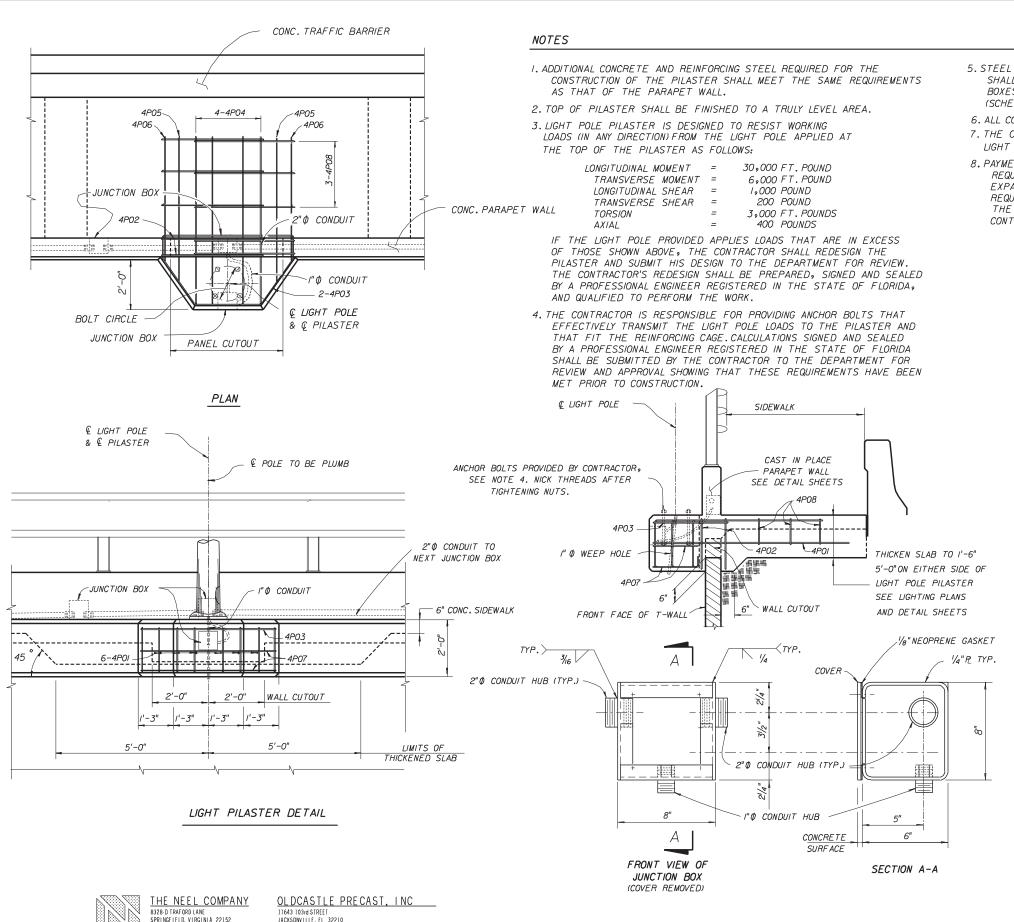


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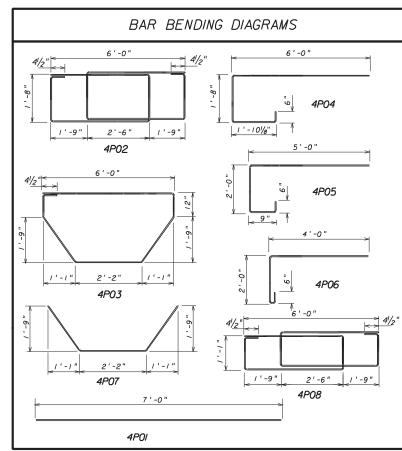
JMC 10/01/98

Checked By



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- 5. STEEL FOR JUNCTION BOXES SHALL CONFORM WITH ASTM-A36. THE BOXES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION. IN LIEU OF STEEL BOXES THE CONTRACTOR MAY SUBMIT FOR APPROVAL MOLDED P.V.C. BOXES (SCHEDULE 80).
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 7. THE COST OF ANCHOR BOLTS SHALL BE INCLUDED IN THE BID PRICE FOR LIGHT POLES
- 8. PAYMENT: THE COST OF ALL LABOR, CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONSTRUCTION OF THE PILASTERS AND ALL CONDUITS. EXPANSION COUPLINGS, JUNCTION BOXES AND MISCELLANEOUS HARDWARE REQUIRED FOR COMPLETION OF THE ELECTRICAL INSTALLATION WITHIN THE LIMITS SHOWN ON THIS SHEET, SHALL BE INCLUDED IN THE CONTRACTOR'S BID PRICE FOR THE MSE WALLS.

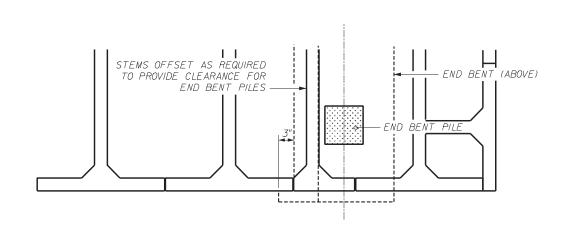


BILL OF REINFORCING STEEL								
MARK	SIZE	NO.REQ'D	LENGTH					
4P0I	4	6	7'-0"					
4P02	4	2	24'-5"					
4P03	4	1	14'-9"					
4P04	4	4	9'-8"					
4P05	4	2	7'-11"					
4P06	4	2	6'-2"					
4P07	4	2	6'-4"					
4P08	4	3	22'-1"					

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

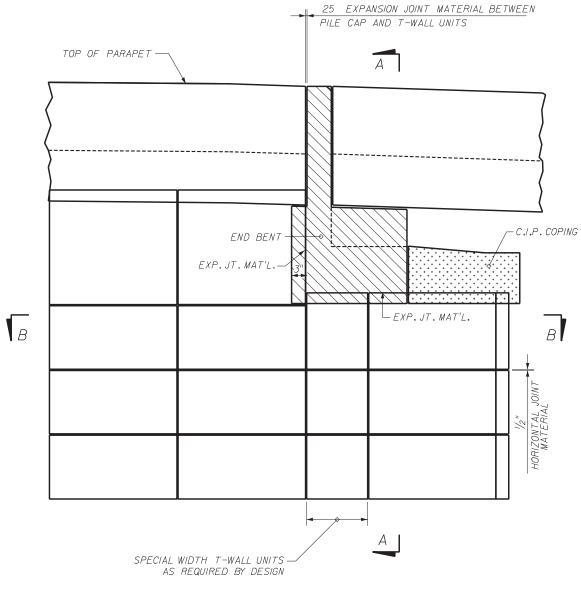
RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3" COVER)

	Names	Dates	Approve	d By	12/1
esigned By	JMC	10/01/98	St	ate Structures D	Oesign Engineer
rawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	JMC	10/01/98	00	11 of 20	5010



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SECTION B-B STEM / END BENT PILE INTERFACE



PART ELEVATION SHOWING
WINGWALL / END BENT INTERFACE

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OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992

SECTION A-A SECTION THRU PILE CAP

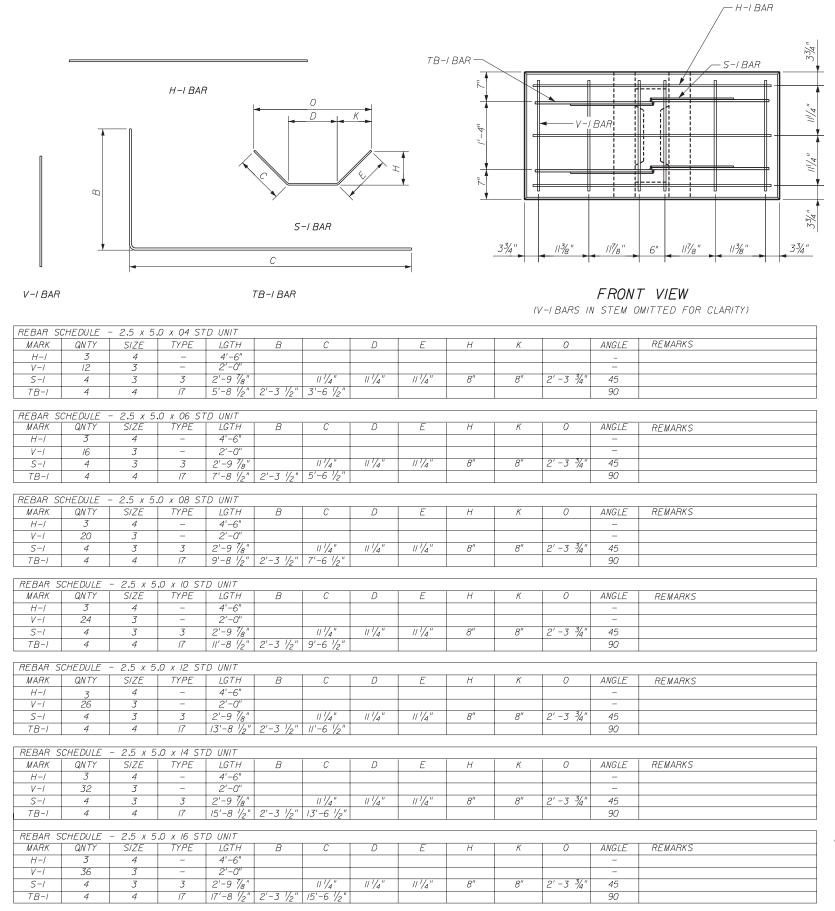
- 1/2" EXP. MATERIAL END BENT WIN. 51/4" FACE OF WINGWALL-

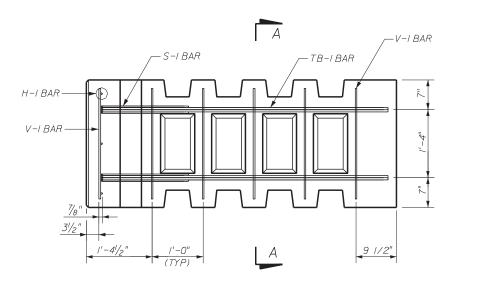
-BRIDGE RAILING AND DECK

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

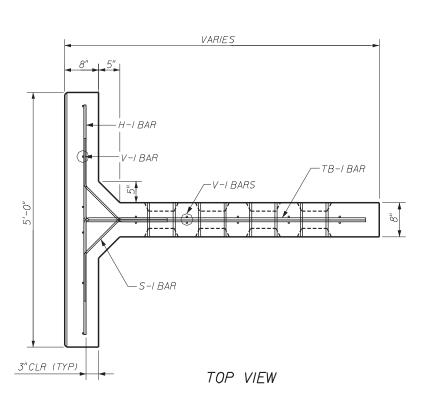
> RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (3" COVER)

	Names	Dates	Approve	d By /)	12/
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Drawn By			Revision	Sheet No.	Index No.
Checked By			00	12 of 20	5010





SIDE VIEW



REINFORCING STEEL - STANDARD UNITS

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 3" MIN. CONCRETE COVER

FX: (904) 778-2992

THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 1703/1913/7858
FX: 1703/1913/7859

OLDCASTLE PRECAST, INC

11643 103rd \$TREE1
JACKSONVILLE, FL 32210
Pte. 1904 778-2990

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

- V−I BARS

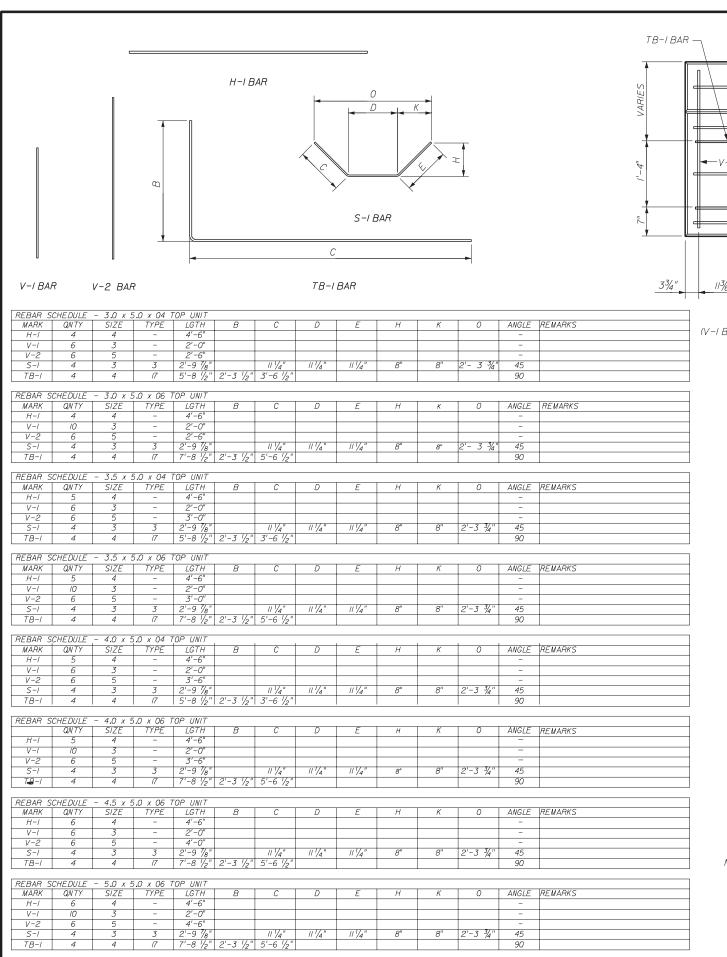
31/8"

SECTION A-A

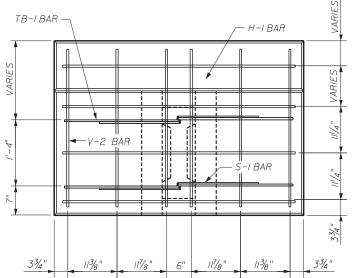
- TB-I BARS

RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3" COVER)

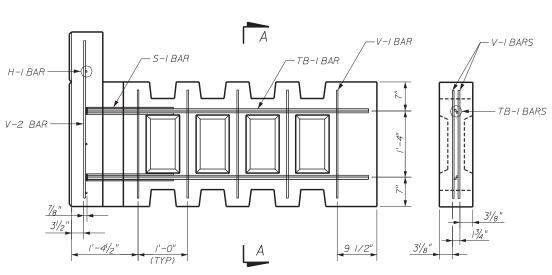
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rawn By	CAA	10/01/98	Revision	Sheet No.	Index No.	
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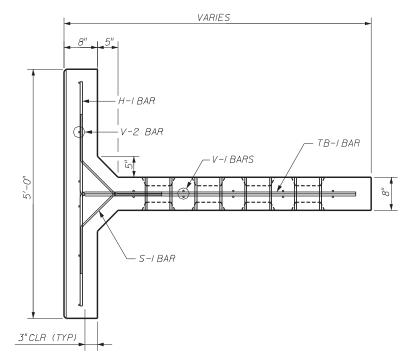
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SIDE VIEW SECTION A-A



TOP VIEW

REINFORCING STEEL - TOP UNITS (I)

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 3" MIN. CONCRETE COVER



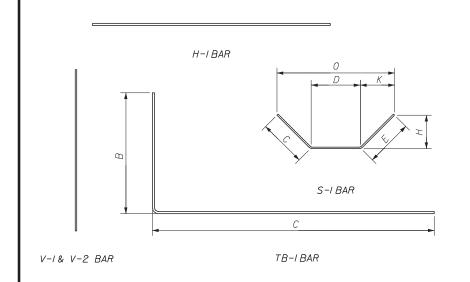
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031) 913-7858
FX: (7031) 913-7859

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 19041 178 2-990
FX: 19041 178-2-999

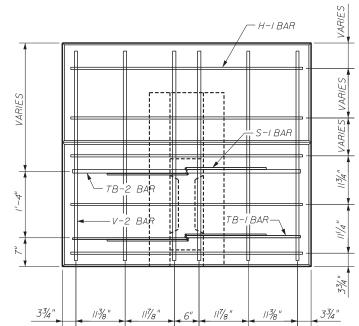
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3"COVER)

	Names	Dates	Approve	d By /) 7	12/
Designed By	JMC	10/01/98	St	ate Structures D	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	JMC	10/01/98	00	14 of 20	5010



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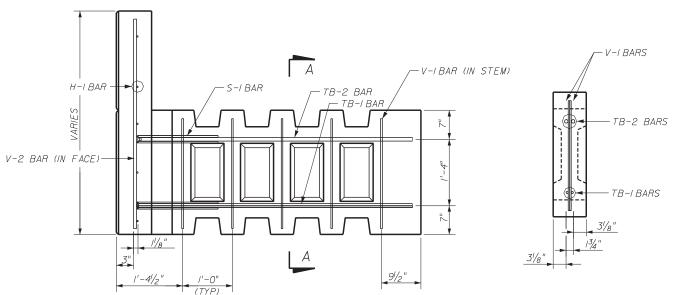
FRONT VIEW (V-I BARS IN STEM OMITTED FOR CLARITY)

REBAR SCH	REBAR SCHEDULE - 5.5 x 5.0 x 08 TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	5'-0"								-	
S-/	4	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 %"	2'-13/4"	45	
TR-I	1	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						an	

REBAR SCH	REBAR SCHEDULE - 6.0 x 5.0 x 08 TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	5'-6"								-	
S-I	4	3	3	2'-9"		9"	1'-3"	9"	5 %"	5 3/8"	2'-13/4"	45	
TB-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	

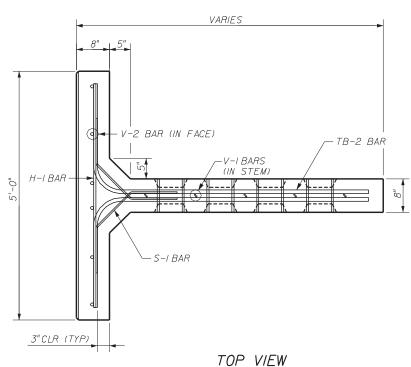
REBAR SCH	REBAR SCHEDULE - 6.5 x 5.0 x 08 TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	6'-0"								-	
S-I	4	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 ¾"	2'-13/4"	45	
TB-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	

REBAR SC	REBAR SCHEDULE - 7.0 x 5.0 x 08 TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	6'-6"								-	
S-I	4	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 %"	2'-13/4"	45	
TB-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	



SIDE VIEW

SECTION A-A



REINFORCING STEEL -TOP UNITS (II)

THESE UNITS WILL ONLY BE USED BY
APPROVAL OF THE F.D.O.T. STRUCTURES DESIGN
OFFICE ON A PROJECT BY PROJECT BASIS.

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 3" MIN. CONCRETE COVER



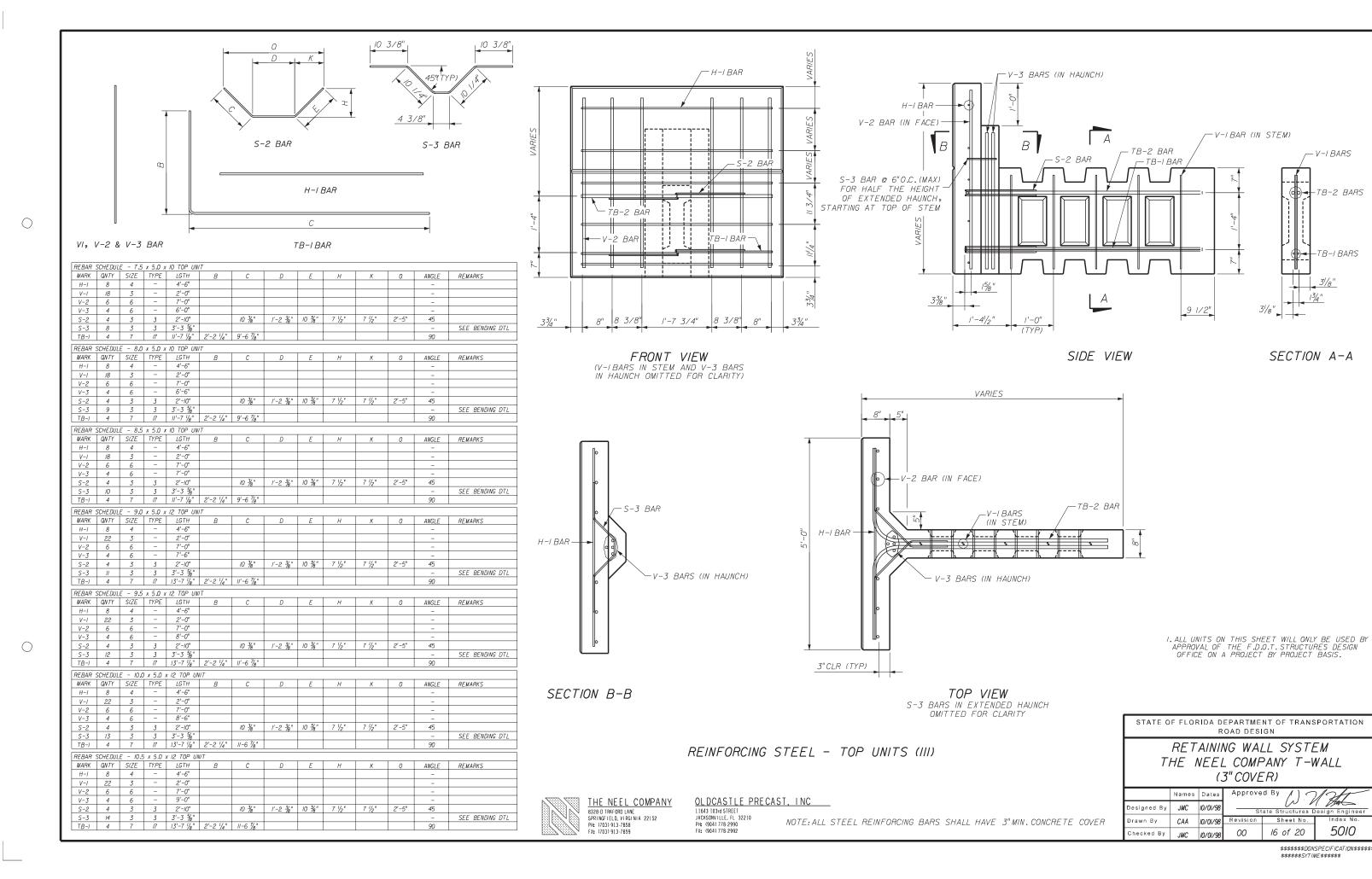
THE NEEL COMPANY
8328-D TRAFORD LAME
SPRINGFIELD, VIRGINIA 22152
PH: (7031 913-7858
FX: (7031 913-7859

OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: 19041 778-2990 FX: 19041 778-2992

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (3" COVER)

	Names	Dates	Approve	d By /) 7	12/1
esigned By	JMC	10/01/98	St	ate Structures D	Design Engineer
rawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
hecked By	JMC	10/01/98	00	15 of 20	5010



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5010

_ V−I BARS

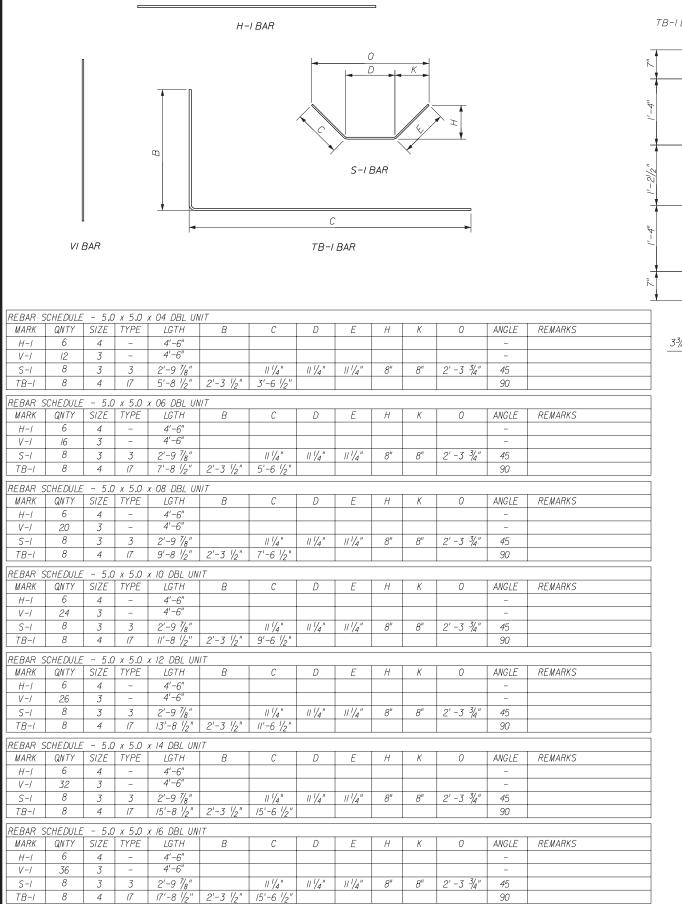
TB-I BARS

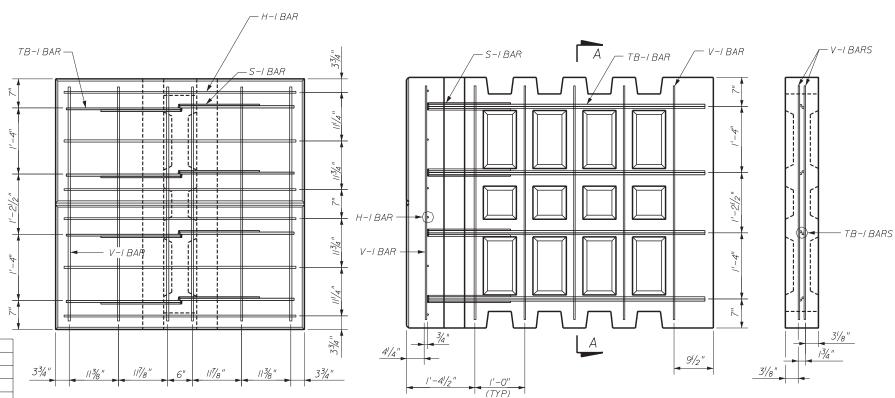
31/8"

13/4"

SECTION A-A

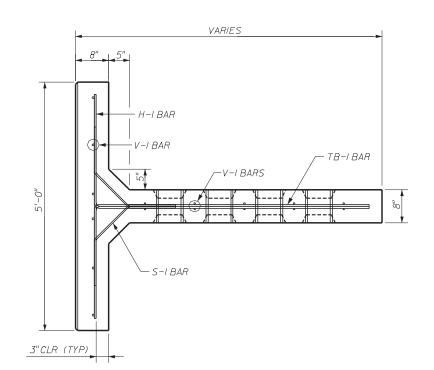
TB-2 BARS





SIDE VIEW

FRONT VIEW
(V-IBARS IN STEM OMITTED FOR CLARITY)



TOP VIEW

REINFORCING STEEL -DOUBLE UNITS



THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17037 913-7858
FX: 17031 913-7859

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSOWILLE, FL 32210
PH: 19041 778 2990
FX: 19041 778 2992

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 3" MIN. CONCRETE COVER

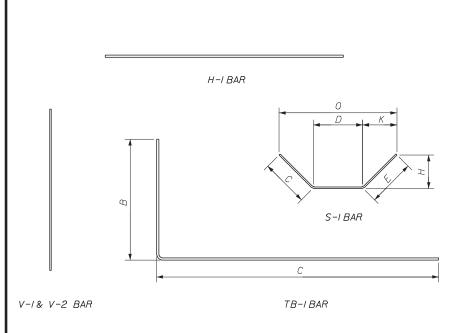
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

SECTION A-A

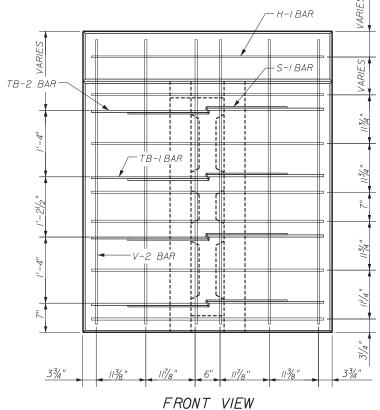
RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(3"COVER)

	Names	Dates	Approve	d By	12/1
Designed By	JMC	10/01/98	St	ate Structures E	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	JMC	10/01/98	00	17 of 20	5010

\$\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$ \$\$\$\$\$\$\$YTIME\$\$\$\$\$\$



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(V-I BARS IN STEM OMITTED FOR CLARITY)

— V−I BARS _ TB-2 BAR – S-I BAR _ TB-I BAR -TB-2 BARS – TB-I BARS H-I BAR — — TB-I BARS V-2 BAR — TB-I BARS 1'-41/2" SECTION A-A SIDE VIEW

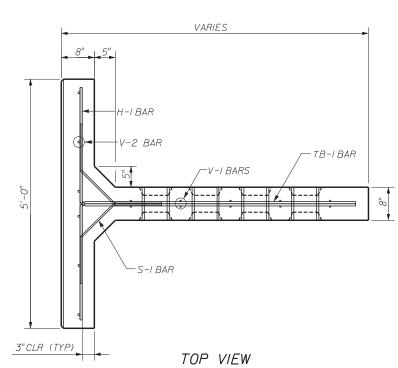
REBAR :	REBAR SCHEDULE - 5.5 x 5.0 x 06 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-/	10	3	-	4'-6"								-	
V-2	6	5	-	5'-0"								_	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
	16 - 16 - 16												

	REBAR SCHEDULE - 6.0 x 5.0 x 06 DBL TOP UNIT													
	MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
	H-I	7	4	-	4'-6"								-	
Г	V-/	10	3	-	4'-6"								-	
	V-2	6	5	-	5'-6"								ı	
	5-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
	TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	

REBAR	REBAR SCHEDULE - 6.5 x 5.0 x 06 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-/	10	3	-	4'-6"								-	
V-2	6	5	-	6'-0"								-	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 ¾"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	

ı	REBAR SCHEDULE - 7.0 x 5.0 x 06 DBL TOP UNIT														
ı	MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS	
ı	H-I	8	4	-	4'-6"								-		
ı	V-/	10	3	-	4'-6"								-		
ı	V-2	6	5	-	6'-6"								-		
ı	S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45		
١	TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90		

REBAR :	REBAR SCHEDULE - 7.5 x 5.0 x 06 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	10	3	-	4'-6"								-	
V-2	6	5	-	7'-0"								ı	
5-/	8	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	



REINFORCING STEEL - DOUBLE TOP UNITS (I)

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 3" MIN. CONCRETE COVER



THE NEEL COMPANY
8328-D TRAFORD LANE
SPRI NGF IELD, VIRGI NIA 22152
Ph. (703) 913-7858
FX: (703) 913-7859

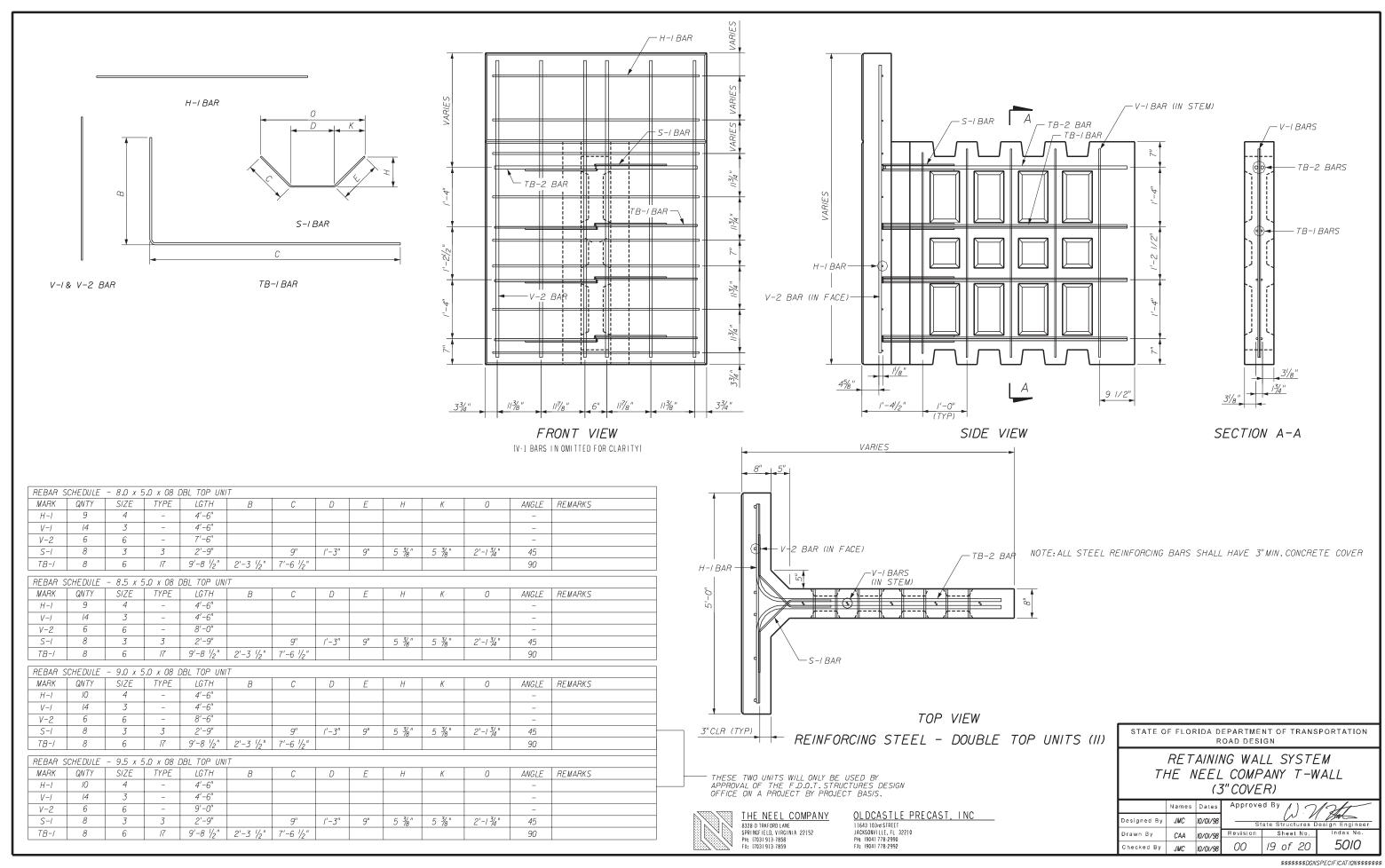
OLDCASTLE PRECAST, INC
11643 103rd STREET
1ACKSONVILLE, FL 32210
PIE 19041 778-2990
FX: 19041 778-2992

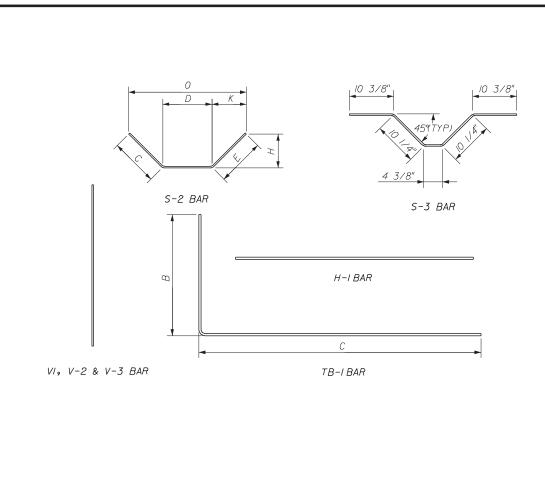
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

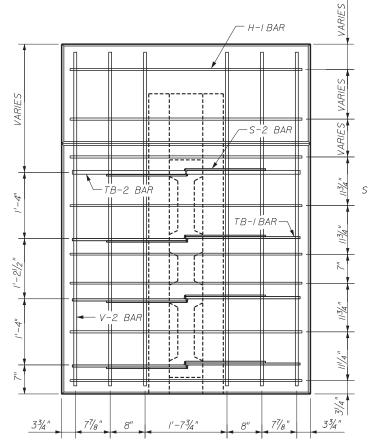
> RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (3" COVER)

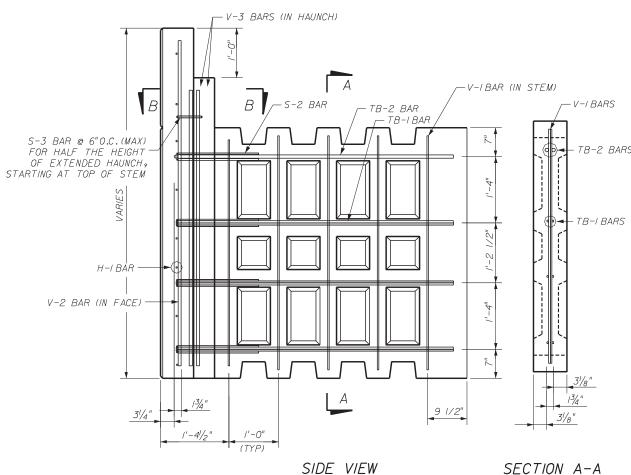
	Names	Dates	Approve	d By /)	12/				
signed By	JMC	10/01/98	State Structures Design Engineer						
awn By	CAA	10/01/98	Revision	Sheet No.	Index No.				
necked By	JMC	10/01/98	00	18 of 20	5010				

\$\$\$\$\$\$\$DGNSPECIFICATION\$









ı	REBAR	SCHEDULE	- 10.0 x	5.0 x 10	DBL TOP ON	/ /								
ı	MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
ı	H-I	//	4	-	4'-6"								-	
ı	V-I	18	3	-	4'-6"								-	
ı	V-2	6	6	-	9'-6"								-	
ı	V-3	4	6	-	8'-6"								-	
ı	S-2	8	3	3	2'-10"		10 3/8"	1'-2 3/8"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
ı	S-3	8	3	3	3'-3 %"								-	SEE BENDING DTL
ı	TB-I	8	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 1/8"						90	

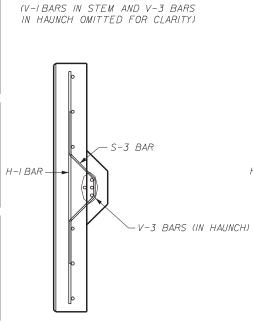
DED 10 COMEDINE 10 0 ... 5 0 ... 10 DDI TOD 11111

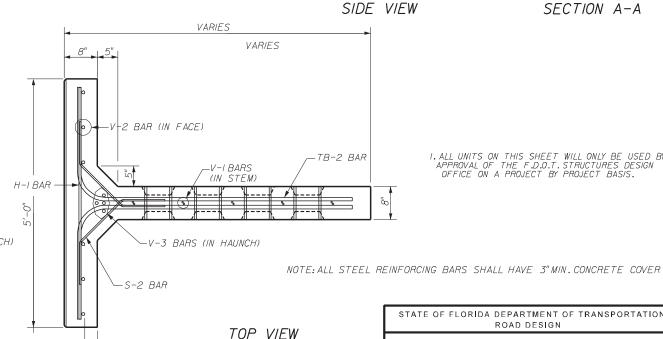
REBAR	REBAR SCHEDULE - 10.5 x 5.0 x 10 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-/	//	4	-	4'-6"								-	
V-I	18	3	-	4'-6"								-	
V-2	6	6	-	10'-0"								-	
V-3	4	6	-	9'-0"								-	
5-2	8	3	3	2'-10"		10 %"	1'-2 %"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
S-3	9	3	3	3'-3 %"								-	SEE BENDING DTL
TB-I	8	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 1/8"						90	

REBAR .	SCHEDULE	- 11.0 x	5.0 x 10 L	DBL TOP UNI	T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	12	4	-	4'-6"								-	
V-I	18	3	-	4'-6"								-	
V-2	6	6	-	10'-6"								-	
V-3	4	6	-	9'-6"								-	
5-2	8	3	3	2'-10"		10 %"	1'-2 3/8"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
S-3	10	3	3	3'-3 %"								-	SEE BENDING DTL
TB-I	8	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 1/8"						90	

- 1	REBAR SCHEDULE - 11.5 x 5.0 x 10 DBL TOP UNIT													
-	MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
-	H-I	12	4	-	4'-6"								-	
-	V-1	18	3	-	4'-6"								-	
- 1	V-2	6	6	-	//'-0"								-	
- 1	V-3	4	6	-	10'-0"								-	
-	S-2	8	3	3	2'-10"		10 3/8"	1'-2 3/8"	10 3/8"	7 1/2"	7 1/2"	2'-5"	45	
- 1	S-3	1/	3	3	3'-3 %"								-	SEE BENDING DTL
_	TB-I	8	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 1/8"						90	

FRONT VIEW





S-3 BARS IN EXTENDED HAUNCH

I. ALL UNITS ON THIS SHEET WILL ONLY BE USED BY APPROVAL OF THE F.D.O.T. STRUCTURES DESIGN OFFICE ON A PROJECT BY PROJECT BASIS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (3" COVER)

Names Dates Approved By / Designed By JMC 10/01/98 CAA 10/01/98 Revision 5010 Checked By JMC 10/01/98 00 | 20 of 20

SECTION B-B

OMITTED FOR CLARITY REINFORCING STEEL - DOUBLE TOP UNITS (III)



THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PHs (703) 913-7858 FX: (703) 913-7859

OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: 19041 778-2990 FX: 19041 778-2992

3"CLR (TYP)

STANDARD DETAILS FOR 2" CONCRETE COVER

T-WALL® RETAINING WALL SYSTEM

DESIGNER



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 \bigcirc

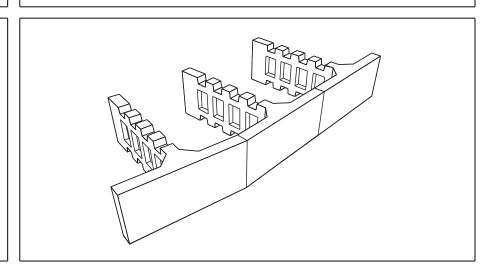
THE NEEL COMPANY

8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER

OLDCASTLE PRECAST, INC.

11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992



MISCELLANEOUS NOTES:

I. DESIGNER:
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VA 22/52
PH: (703) 9/3-7858
FX: (703) 9/3-7859

2. PRECASTER:

OLDCASTLE PRECAST INC.

II643 IO3rd STREET

JACKSONVILLE, FL 322IO

PH: (904) 778-2990

FX: (904) 778-2992

3. MATERIALS SUPPLIED BY PRECASTER:
-PRECAST T-WALL UNITS
-PRECAST SHEAR KEYS
-HORIZONTAL JOINT MATERIAL
-VERTICAL JOINT MATERIAL AND ADHESIVE
-SHEAR KEY JOINT MATERIAL

DESIGN NOTES:

I. DESIGN IS BASED ON THE ASSUMPTION
THAT THE MATERIAL WITHIN THE
RETAINING WALL VOLUME, METHODS OF
CONSTRUCTION, AND QUALITY OF
PREFABRICATED MATERIALS SHALL CONFORM
TO SPEC SECTION 548 - RETAINING WALL
SYSTEMS.

2. SOIL PARAMETERS:

-SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS
OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE
WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN
PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL
SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUE OF \$\phi\$,
C AND GAMMA SHALL BE PROVIDED IN THE SHOP DRAWINGS

3. FACTORS OF SAFETY:
-OVERTURNING - 2.0
-SLIDING - 1.5
-INTERNAL PULLOUT - 1.5
-BEARING CAPACITY - 2.5
-OVERALL STABILITY - 1.5

4. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY THE OWNER. ON THE BASIS OF THIS INFORMATION, THE NEEL COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN, INCLUDING FOUNDATION AND SLOPE STABILITY, IS THE RESPONSIBILITY OF OTHERS.

5. PANELS WITH CANTILEVERED (EXTENDED) FACE SHALL ONLY BE USED TO AVOID OBSTRUCTIONS AS APPROVED ON THE SHOP DRAWINGS.

MATERIALS NOTES:

I. PRECAST CONCRETE:

-PRECAST T-WALL UNITS - PER SPEC
SECTION 548

-PRECAST SHEAR KEYS - PER SPEC
SECTION 548

2. C.I.P.CONCRETE:

-C.I.P.LEVELING PAD - PER SPEC
SECTION 548

-OTHER C.I.P.CONCRETE - PER SPEC SECTION 548

3. REINFORCING STEEL:
-PER SPEC SECTION 548

4. JOINT MATERIAL:

-HORIZONTAL JOINT FILLER:

-I/2" x 4" x 5'-0"

-PREFORMED EPDM

-DUROMETER: 80 - 90

-VERTICAL JOINT COVER:

-TENSAR DC4205 OR EQUAL

-I2" WIDE x HEIGHT OF JOINT

-GEOCOMPOSITE MEETING REQUIREMENTS

OF SPEC SECTION 548

-SHEAR KEY WRAP:

-I/4" x 8" x 24"

-AVI ASTRO-FOAM AF-250

5. BACKFILL:
-PER SPEC SECTION 548

DESIGNER:



THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER:

OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992

- I. ALL CONSTRUCTION PROCEDURES SHALL COMPLY WITH
 SPEC SECTION 548 AND THE "T-WALL CONSTRUCTION
 MANUAL" (PROVIDED BY THE NEEL COMPANY OR OLDCASTLE
 PRECAST, INC). IN THE EVENT OF A DISCREPANCY BETWEEN
 THE SPEC AND THE "T-WALL CONSTRUCTION MANUAL", THE
 SPEC SHALL CONTROL.
- 2. FOR LOCATION AND ALIGNMENT OF T-WALL STRUCTURE, SEE RETAINING WALL CONTROL PLANS.

CONSTRUCTION NOTES:

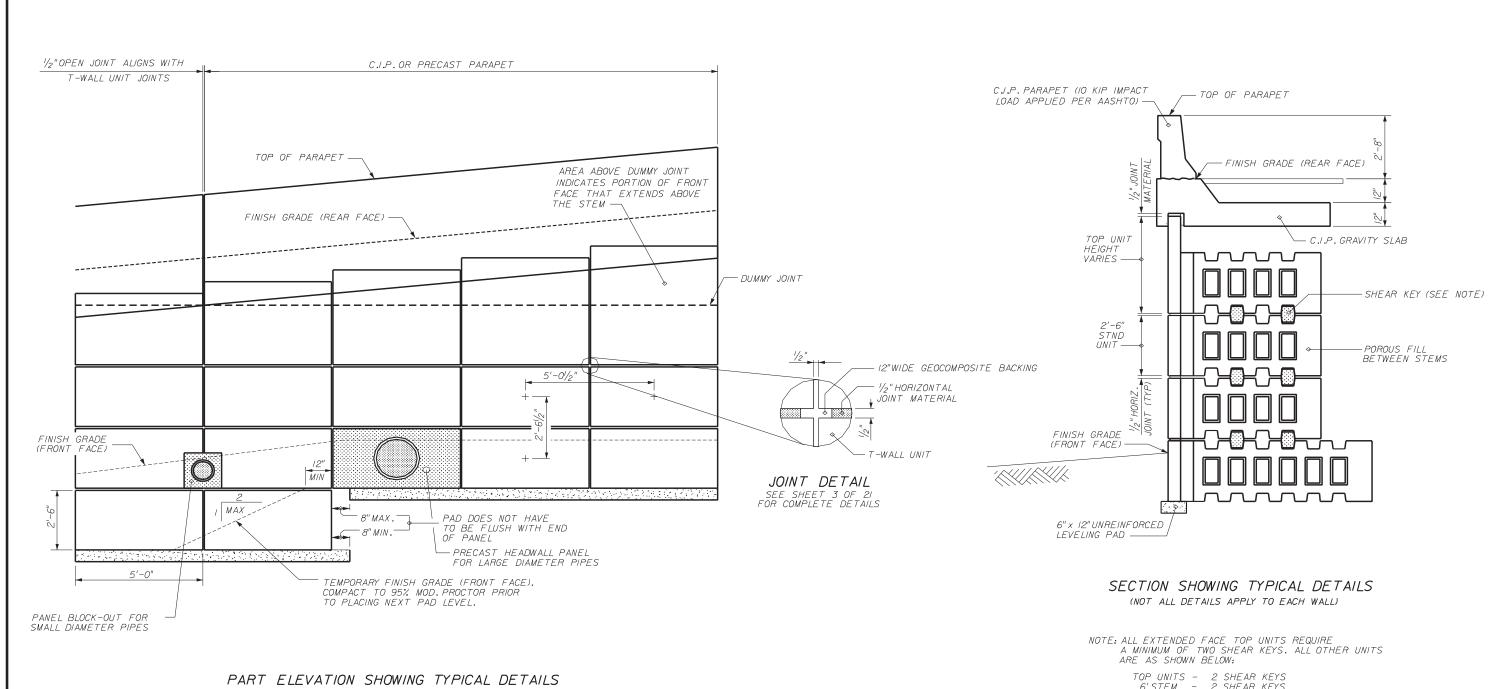
- 3. T-WALL STRUCTURES ON CURVES SHALL BE BUILT IN CHORDS AS SHOWN IN THE T-WALL DESIGN DRAWINGS.
- 4. IF MANHOLES OR DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN IN THE T-WALL DESIGN DRAWINGS.
- 5. IF PILES ARE LOCATED WITHIN THE RETAINING WALL VOLUME, THEY SHALL BE DRIVEN BEFORE CONSTRUCTION OF THE T-WALL STRUCTURE.
- 6. T-WALL UNITS SHALL BE PLACED ONE ROW AT A TIME, AND BACKFILLED BEFORE PLACEMENT OF THE NEXT ROW.
- 7. IF A STRUCTURE EXCEEDS 20'IN HEIGHT, THE FINISH GRADE AT THE FACE OF THE WALL SHALL BE PLACED AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS 20'IN HEIGHT.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR CONTROLLING STORM
 WATER DRAINAGE IN THE VICINITY OF THE WALL DURING
 CONSTRUCTION. STORMWATER RUNOFF SHALL BE COLLECTED
 AND DISCHARGED AWAY FROM THE WALL AND THE RETAINING WALL VOLUME.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approved By						
Designed By	J MC	10/01/98	State Structures Design Engineer						
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.				
Checked By	J MC	10/01/98	00	1 of 21	5011				

THIS SYSTEM SHALL BE USED IN MODERATELY OR SLIGHTLY AGGRESSIVE ENVIRONMENTS.



(NO SCALE)

 \bigcirc

 \bigcirc

6'STEM - 2 SHEAR KEYS 8'STEM - 2 SHEAR KEYS 8 STEM - 2 SHEAR KETS 10'STEM - 2 SHEAR KEYS 12'STEM - 2 SHEAR KEYS 14'STEM - 3 SHEAR KEYS 16'STEM - 3 SHEAR KEYS

> STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

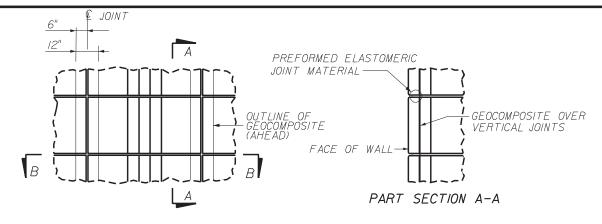
L		Names	Dates	Approve	^{а ву} /) : 7	12/1
l	Designed By	J MC	10/01/98	St	ate Structures E	Design Engineer
ı	Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Ì	Checked By	J MC	10/01/98	00	2 of 21	5011

DESIGNER:

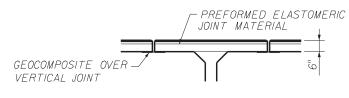


THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER: OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: 1904) 778-2990 FX: 1904) 778-2992



PART ELEVATION - REAR FACE

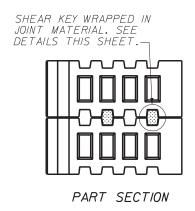


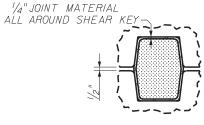
PART SECTION B-B

NOTES:

- 2. VERTICAL JOINT:
 //2"SPACE
 12"WIDE GEOCOMPOSITE BACKING, CENTERED
 ABOUT JOINT CENTERLINE.

JOINT MATERIAL DETAILS





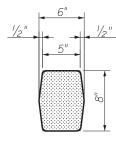
SHEAR KEY / JOINT MATERIAL ARRANGEMENT

NOTES:

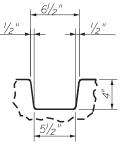
 \bigcirc

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- I. SHEAR KEY JOINT MATERIAL: MINIMUM OF ONE'/4" x 8" x 24" PIECE OF AVI ASTRO-FOAM AF-250 PER SHEAR KEY.
- 2. JOINT MATERIAL MAY BE ADDED OR REMOVED TO AID IN SHIMMING AND ALIGNING, HOWEVER SHEAR KEY MUST FIT SNUG IN THE SHEAR KEY BLOCKOUT WHEN UNIT IS IN ITS FINAL POSITION.
- 3. MINIMUM OF 2 SHEAR KEYS REQUIRED PER UNIT. SEE NOTES ON SHEET 2 OF 21, 'TYPICAL DETAILS (1)'.







SHEAR KEY BLOCKOUT DIM's

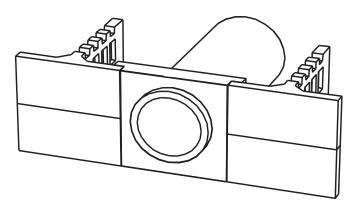
SHEAR KEY DETAILS

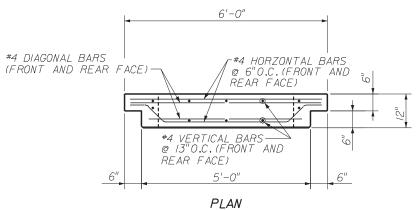
DESIGNER:

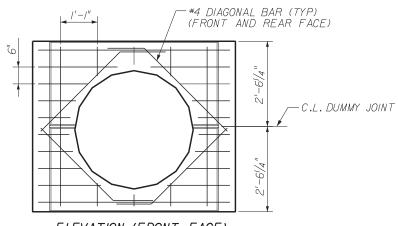


PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 1904/17/8-2990







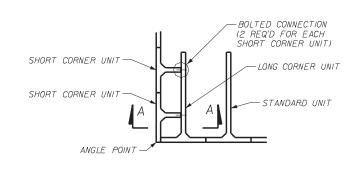
ELEVATION (FRONT FACE)

PRECAST HEADWALL PANEL
FOR LARGE DIAMETER PIPES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY T-WALL
(2" COVER)

	Names	Dates	Approve	d By /)	12/1
esigned By	J MC	10/01/98	st	ate Structures E	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	3 of 21	<i>5011</i>



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LONG CORNER UNIT

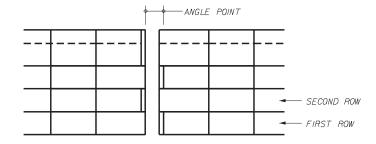
ANGLE POINT

BOLTED CONNECTION
(2 REQ'D FOR EACH
SHORT CORNER UNIT)

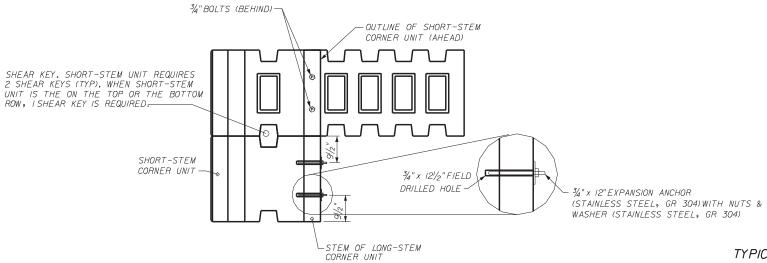
STANDARD UNIT
SHORT CORNER UNIT

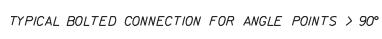
PART PLAN - FIRST ROW

PART PLAN - SECOND ROW



PART ELEVATION





- ¾"x 4"GALVANIZED EXPANSION BOLT AND WASHER

- ⅔"x 2"(LENGTH AS REQ'D) GALVANIZED BENT PLATE

-BOLTED CONNECTION

SHORT STEM UNIT

VARIES

LONG STEM UNIT -

GEOCOMPOSITE BACKING (TYP)

TYPICAL ANGLE S

PART PLAN - ANGLE > 90°

SHORT AND LONG STEMS ALTERNATE
PER 90°CORNER DETAIL

В

TYPICAL ANGLE POINT DETAIL

FIELD LOCATED AND DRILLED HOLE IN T-WALL STEM APPROXIMATE LOCATION
OF TB-I BARS IN STEM

ADDITIONAL VERTICAL *4 BAR

TWO CONNECTIONS
PER SHORT STEM

VIEW B-B

TYPICAL CORNER UNIT ARRANGEMENT

STEM LENGTHS VARY - SEE SPECIFIC

ELEVATIONS FOR PROPER UNITS

NO SCALE

SECTION A-A

DESIGNER:



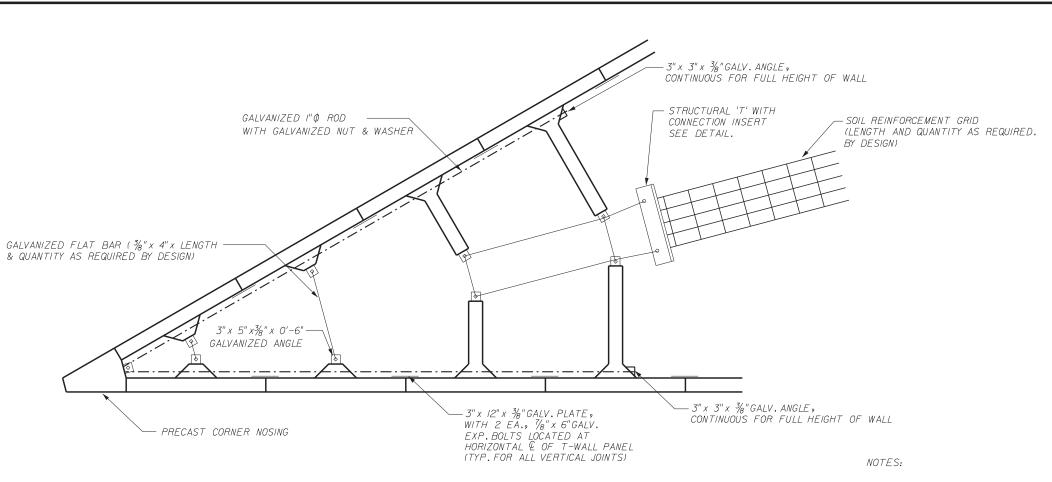
PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 19041 778 2990
FL: 19041 778 2992

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

	Names	Dates	Approve	d By /) 7	12/1				
esigned By	J MC	10/01/98	State Structures Design Engineer						
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.				
Checked By	J MC	10/01/98	00	4 of 21	l <i>5011</i>				



BENT BAR WELDED
TO ENDS OF FLANGE

I" x 3" GALVANIZED BOLT,
WASHERS AND NUT

FLAT BAR
GALVANIZED

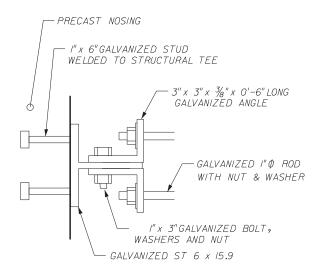
OF FLANGE

ST 6 x 15.9 WITH
CONNECTION INSERT
WELDED TO FLANGE.

SOIL GRID CONNECTION DETAIL SEVERE ACUTE CORNERS

PART PLAN SEVERE ACUTE ANGLE DETAIL ANGLE 45°OR LESS

 \bigcirc



ROD/NOSING CONNECTION DETAIL

- SOIL GRID TO BE DESIGNED FOR PULLOUT & TENSION.
 QUANTITY AND LENGTH OF GRIDS TO BE AS REQUIRED BY DESIGN.
- 2. CONNECTION INSERT:
 - -PER SPEC SECTION 548
 - -WI/WIRE
 - -WELDED PER ASTM A185 PRIOR TO GALVANIZATION
- 3. LOCKING BAR:
 - -PER SPEC SECTION 548
- 4. SOIL REINFORCEMENT GRIDS:
 - -PER SPEC SECTION 548
 - -WIIWELDED WIRE GRIDS:
 -5 LONGITUDINAL WIRES @ 6"O.C., LENGTH AS REQUIRED BY DESIGN
 - -24" LONG TRANSVERSE BARS AT 6" OR 12" O.C., AS REQUIRED BY DESIGN -SOIL GRID LENGTHS SHOWN ON T-WALL DESIGN DRAWINGS ARE NOMINAL LENGTHS AS REQUIRED BY DESIGN CALCULATIONS. DUE TO MANUFACTURING

TOLERANCES, ACTUAL GRID LENGTHS MAY BE LONGER.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approved By State Structures Design Engineer						
signed By	J MC	10/01/98							
awn By	CAA	10/01/98	Revision	Sheet No.	Index No.				
ecked By	J MC	10/01/98	00	5 of 21	5011				

DESIGNER:

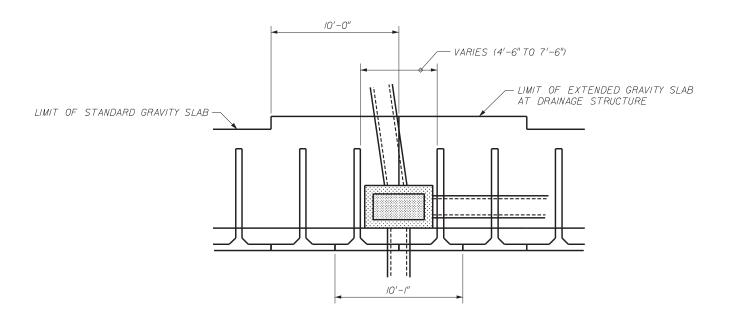


THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17031 913-7858
FX: 17031 913-7859

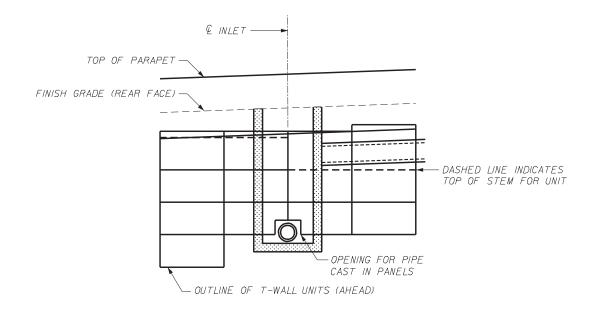
PRECASTER:

OLDCASTLE PRECAST, INC

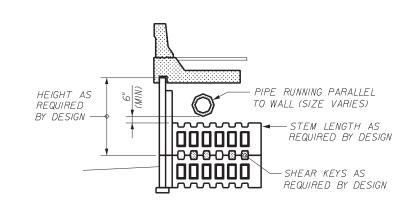
11643 103rd STREET JACKSONVILLE, FL 32210 PH: [904] 778-2990 FX: [703] 913-7859



PART PLAN



PART ELEVATION (FRONT FACE)



SECTION
(SHOWING PIPE PARALLEL TO WALL)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

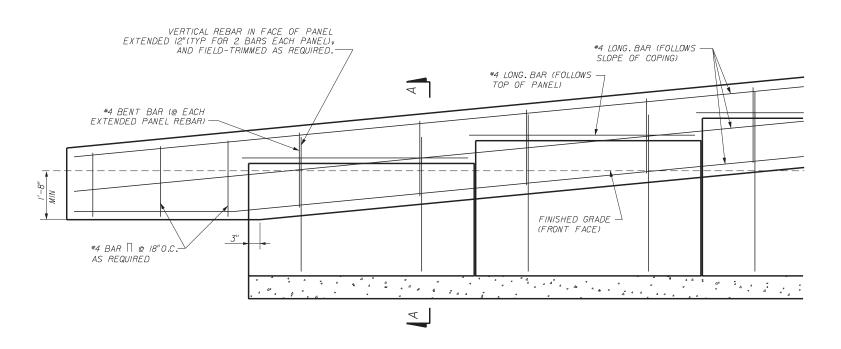
DESIGNER:



THE NEEL COMPANY
8328-0 TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (703) 913-7858
FX: (703) 913-7859

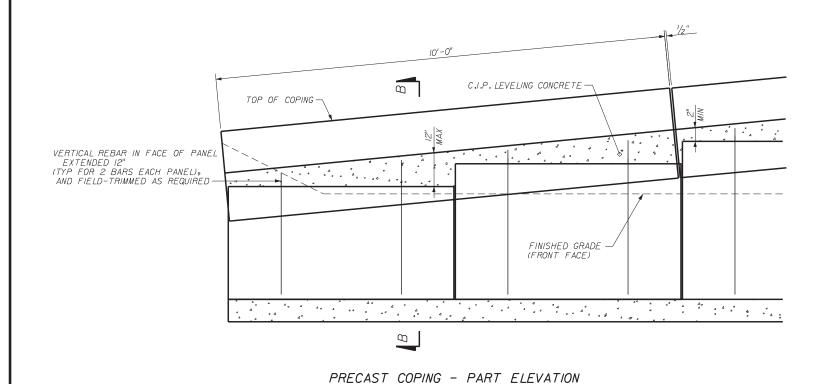
OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSOWILLE, FL 32210
Pkt 19941 1782-2990
FX: 19041 1782-2992

PRECASTER:



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C.I.P. COPING TREATMENT AT BEGINNING/END OF WALLS



*4 LONG. BAR TO FOLLOW

T-WALL FACE REBAR EXTENDED

12" ABOVE TOP OF PANEL;

FIELD-TRIMMED AS REQ'D

*4 BENT BAR

C.I.P. COPING

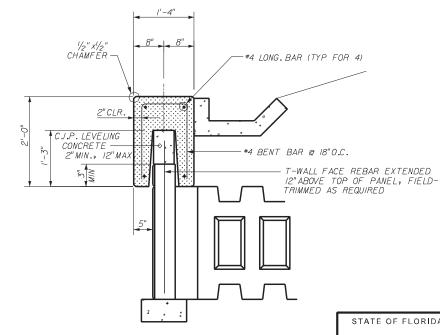
REBAR DETAILS

BENT BAR TO FOLLOW

AS REQUIRED TO PROVIDE MIN OF 2"

OF CONCRETE COVER

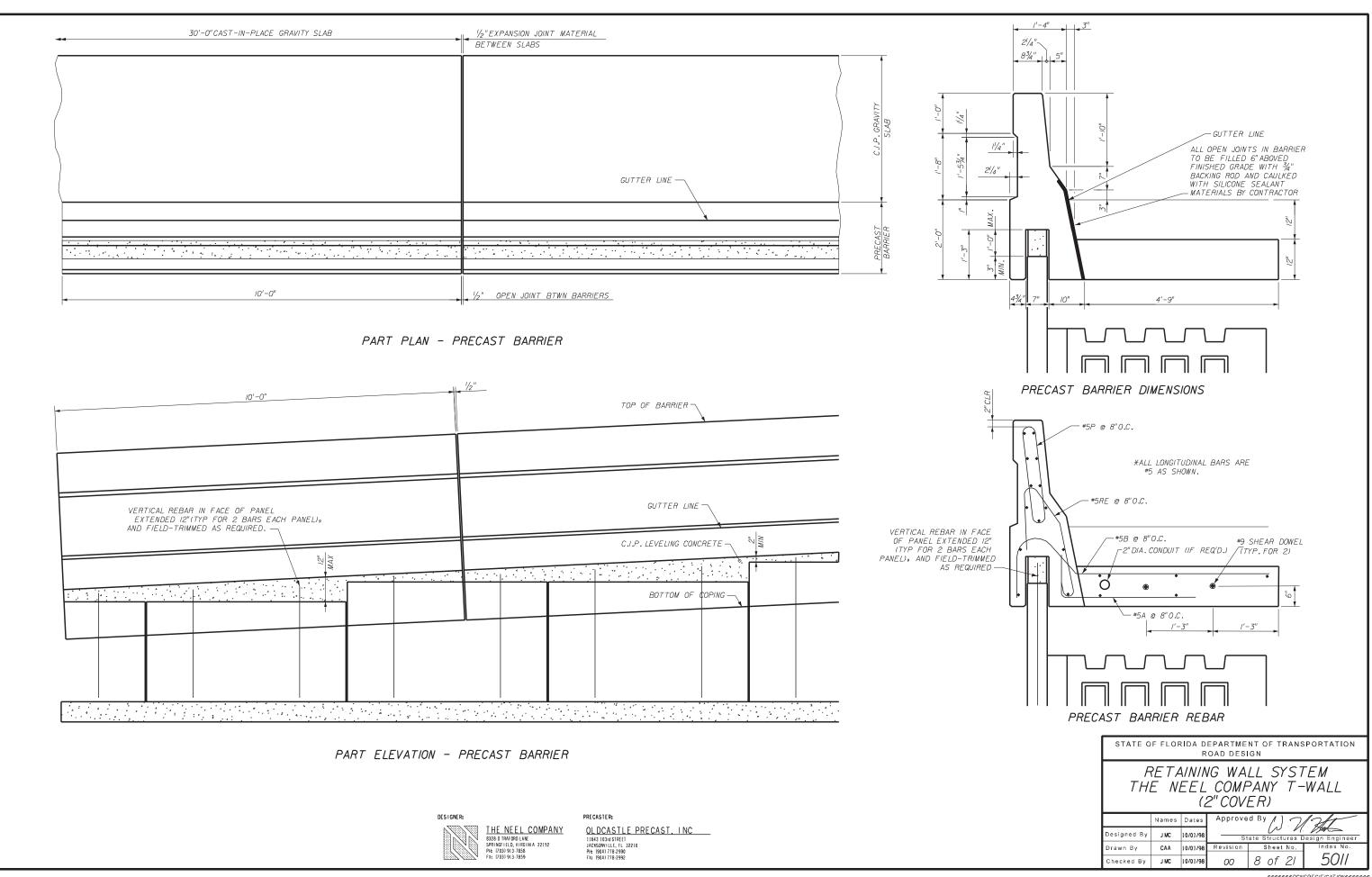
SECTION A-A C.J.P. COPING



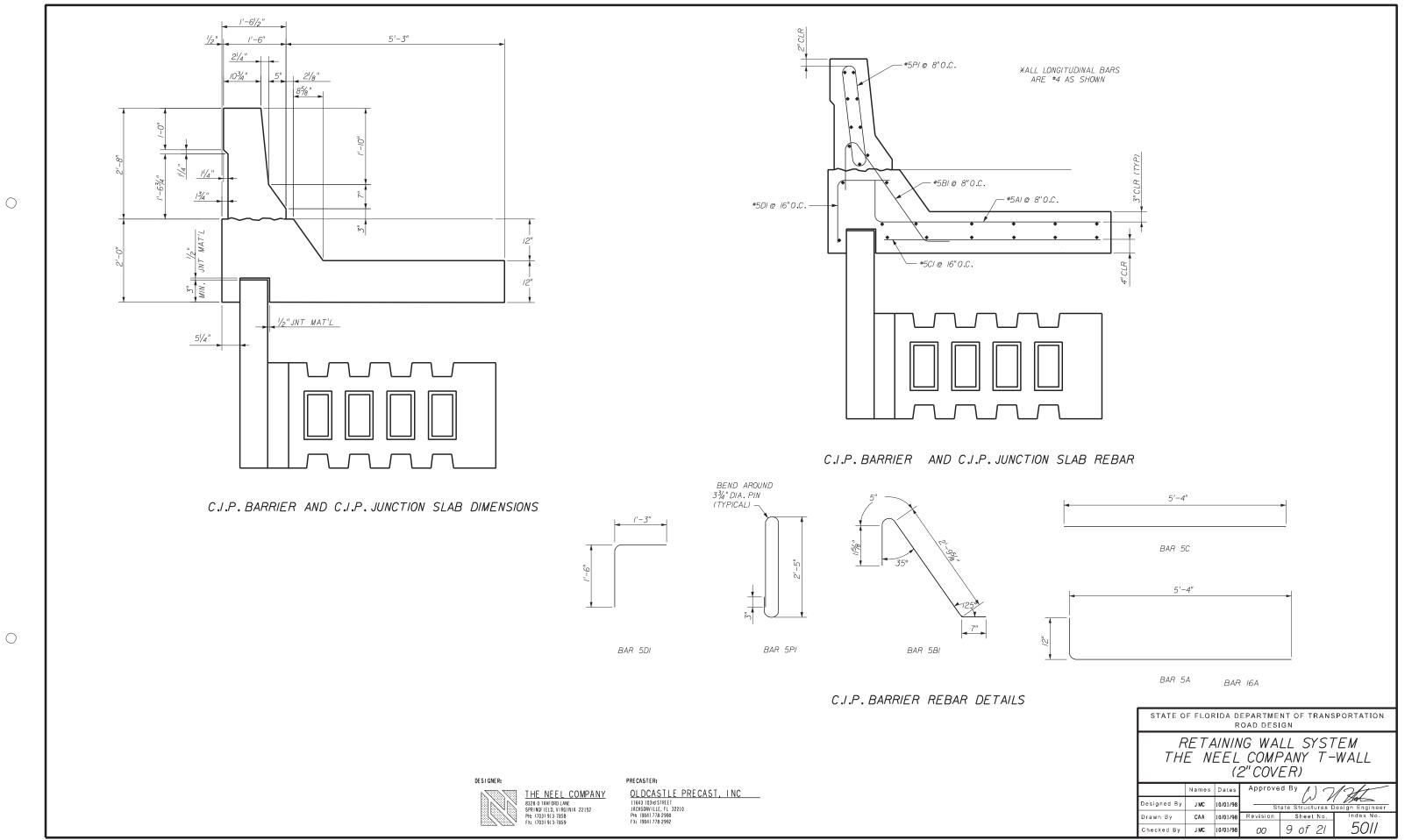
SECTION B-B PRECAST COPING STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

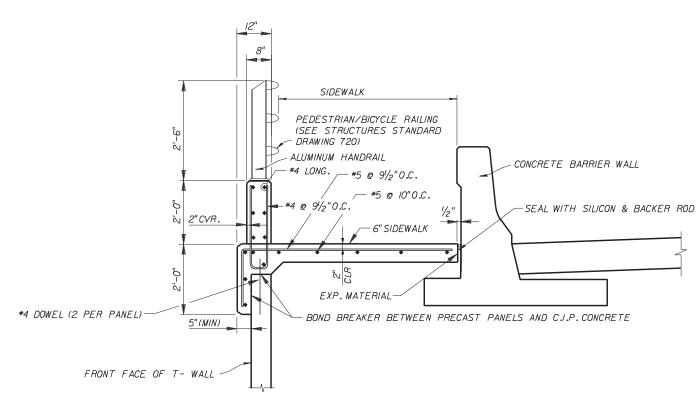
RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approve	^{а ву} /) . ?	12/
igned By	J MC	10/01/98		W VI	- Dun -
	55	10.01.50	St	ate Structures D	esign Engineer
wn By	CAA	10/01/98	Revision	Sheet No.	Index No.
cked By	J MC	10/01/98	00	/ ot 21	<i>' </i>



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C.I.P. PARAPET DETAIL W/ HANDRAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approve	d By	124T
Designed By	J MC	10/01/98	St	ate Structures E	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	10 of 21	5011

DESIGNER:

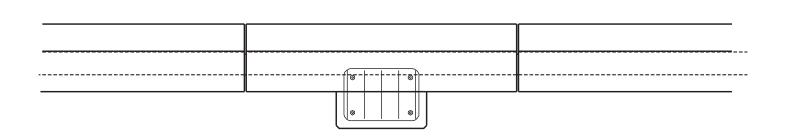
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THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (7031 913-7858 FX: (7031 913-7859

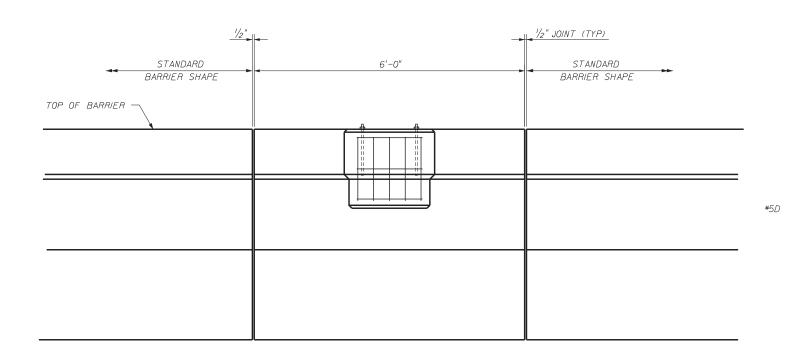
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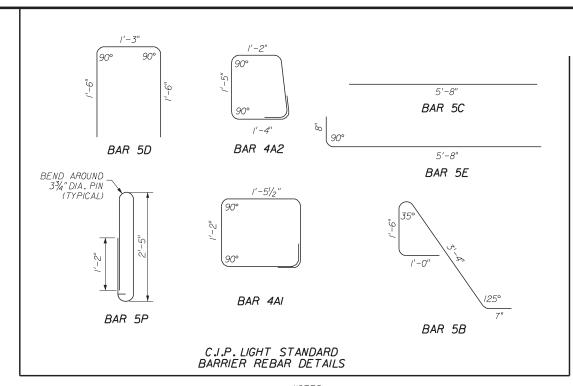
OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONWILLE, FL 32210
PH: 19041 778-2990
FX: 19041 778-2992

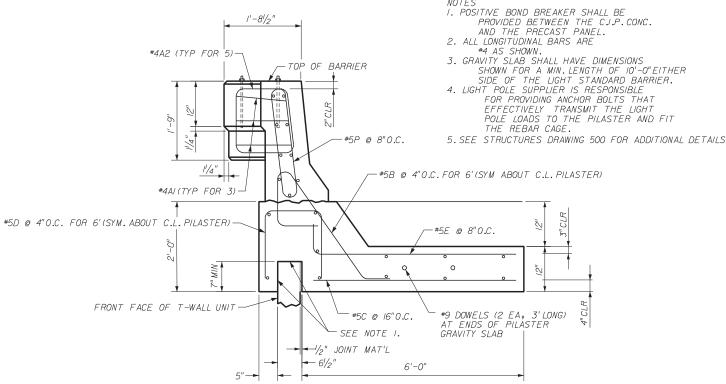


C.I.P. LIGHT STANDARD BARRIER - PART PLAN WITH REBAR (BARRIER AND GRAVITY SLAB REBAR OMITTED FOR CLARITY)

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C.J.P. LIGHT STANDARD BARRIER - PART SECTION WITH REBAR

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN C.I.P. LIGHT STANDARD BARRIER - PART ELEVATION

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

Names Dates Designed By J MC 10/01/9 CAA 10/01/98 Checked By J MC 10/01/98 11 of 21 00

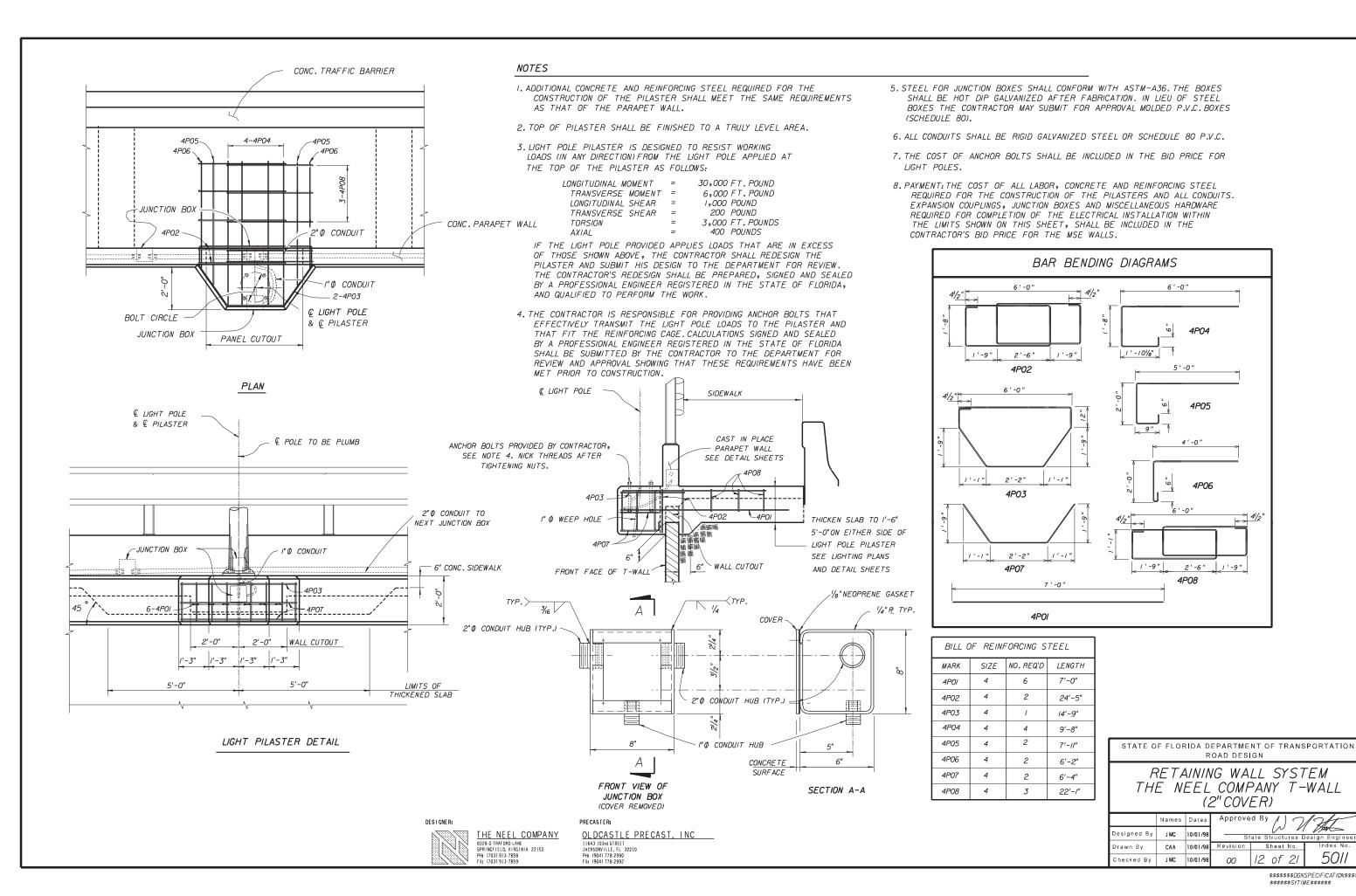
(BARRIER AND GRAVITY SLAB REBAR OMITTED FOR CLARITY)

DESIGNER:

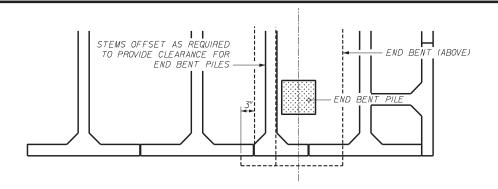
THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: 1703/913-7858 FX: 1703/913-7859

OLDCASTLE PRECAST, INC 11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992

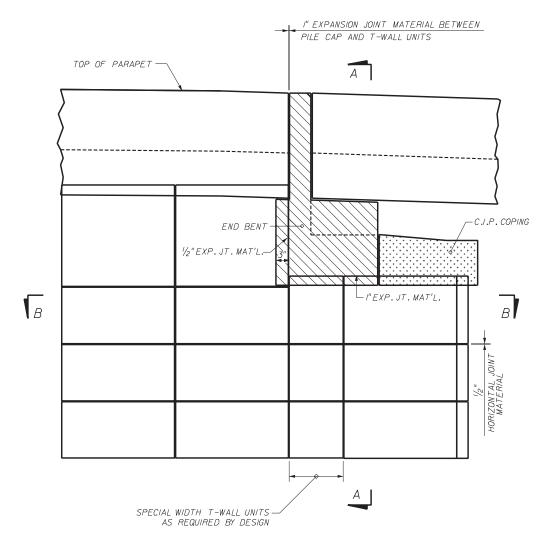
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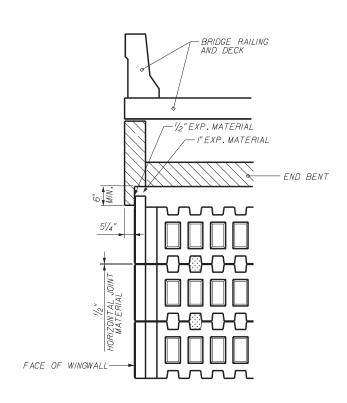
12 of 21



SECTION B-B STEM / END BENT PILE INTERFACE



PART ELEVATION SHOWING
WINGWALL / END BENT INTERFACE

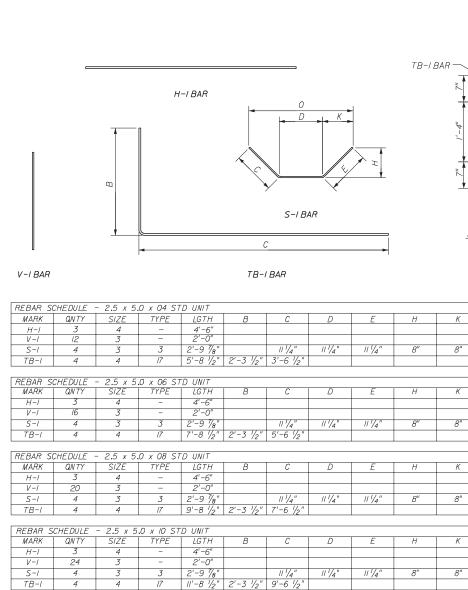


SECTION A-A SECTION THRU PILE CAP

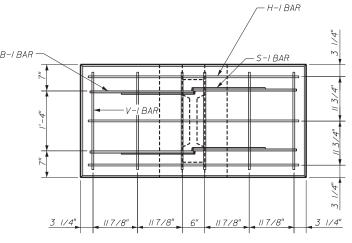
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

	Names	Dates	Approve	d By /) 7	12/
Designed By	J MC	10/01/98	St	ate Structures D	esign Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	13 of 21	<i>5011</i>



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FRONT VIEW
(V-IBARS IN STEM OMITTED FOR CLARITY)

Н	K	0	ANGLE	REMARKS	
			-		
			-		
8"	8"	2'-3 3/4"	45		

	REBAR S	CHEDULE	- 2.5 x 5	.0 x 06 S	TD UNIT									
ſ	MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	E	Н	K	0	ANGLE	REMARKS
	H-I	3	4	-	4'-6"								-	
	V-/	16	3	-	2'-0"								-	
	5-1	4	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
	TB-I	4	4	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	

REBAR SO	CHEDULE :	- 2.5 x 5	.0 x 08 S	TD UNIT									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	3	4	-	4'-6"								-	
V-/	20	3	-	2'-0"								_	
S-/	4	3	3	2'-9 7/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	4	4	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	

REBAR S	CHEDULE	- 2.5 x 5	.0 x 10 S7	D UNIT									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	3	4	-	4'-6"								-	
V-/	24	3	-	2'-0"								-	
5-/	4	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	4	4	17	11'-8 1/2"	2'-3 1/2"	9'-6 1/2"						90	

REBAR S	CHEDULE	- 2.5 x 5	.0 x 12 S7	D UNIT										
MARK QNTY SIZE TYPE LGTH B C D E H K O ANGLE REMARKS														
H-I	3	4	-	4'-6"								-		
V-/	26	3	-	2'-0"								-		
S-I	4	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45		
TB-I	4	4	17	13'-8 1/2"	2'-3 1/2"	11'-6 1/2"						90		

REBAR S	CHEDULE	- 2.5 x 5	.0 x 14 ST	D UNIT									
MARK QNTY SIZE TYPE LGTH B C D E H K O ANGLE REMARKS													
H-I	3	4	-	4'-6"								-	
V-/	32	3	-	2'-0"								-	
S-I	4	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	4	4	17	15'-8 1/2"	2'-3 1/2"	13'-6 1/2"						90	

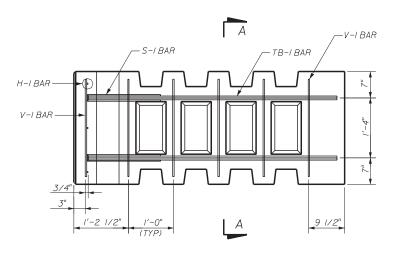
REBAR S	CHEDULE	- 2.5 x 5	.0 x 16 ST	TD UNIT									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-/	3	4	-	4'-6"								-	
V-/	36	3	-	2'-0"								-	
5-1	4	3	3	2'-9 7/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	<i>4</i> 5	
TB-I	4	4	17	17'-8 1/2"	2'-3 1/2"	15'-6 1/2"						90	

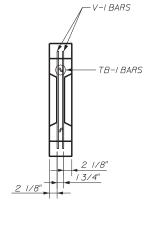




THE NEEL COMPANY
8328-D TRAFORD LANE
SPRI NGF IELD, VI RGINIA 22152
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FX: (703) 913-7859

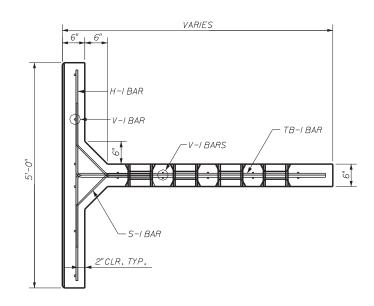
PRECASTER: OLDCASTLE PRECAST, INC
11643 (03rd \$18EET)
JACKSOWLILE, FL 32210
Phe 19041 778-2990
Fx 19041 778-2992





SIDE VIEW

SECTION A-A

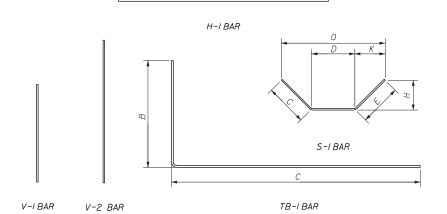


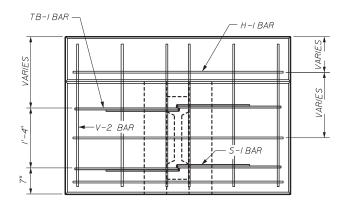
TOP VIEW REINFORCING STEEL - STANDARD UNITS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

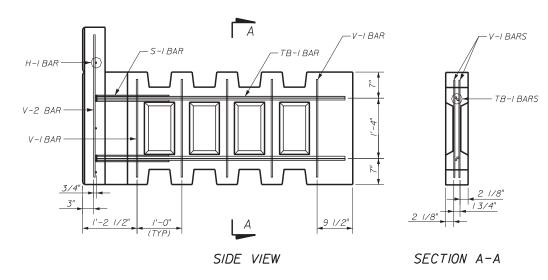
RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

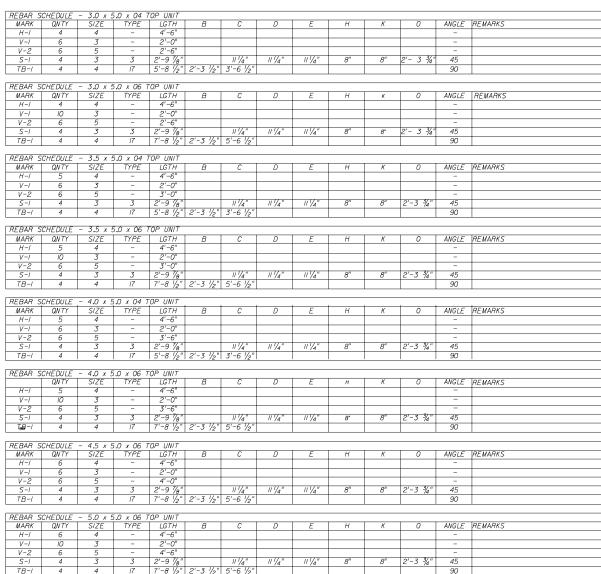
	Names	Dates	Approve	d By /)	12/
Designed By	J MC	10/01/98	St	tate Structures E	esign Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	14 of 21	5 <i>011</i>

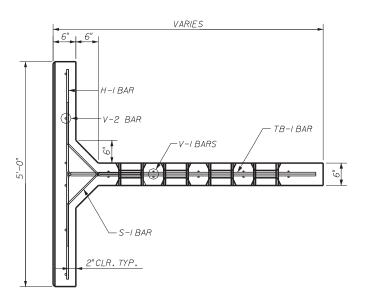




FRONT VIEW (V-IBARS IN STEM OMITTED FOR CLARITY)







TOP VIEW

REINFORCING STEEL - TOP UNITS (I)

MARK	QNII	SIZE	ITPE	LG I H	B	(U		11	Λ.	0	ANGLE	MEMARKS
H-I	6	4	-	4'-6"								-	
V-/	6	3	-	2'-0"								-	
V-2	6	5	-	4'-0"								-	
S-I	4	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	4	4	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
										•			
REBAR S	CHEDULE	- 5.0 x 5	5.0 x 06 7	OP UNIT									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-/	6	4	-	4'-6"								-	
V-/	10	3	-	2'-0"								-	
V-2	6	5	-	4'-6"								-	
S-I	4	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	4	4	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
									-				

DESIGNER:



THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: 1703) 913-7858 FX: 1703) 913-7859

PRECASTER:

OLDCASTLE PRECAST, INC

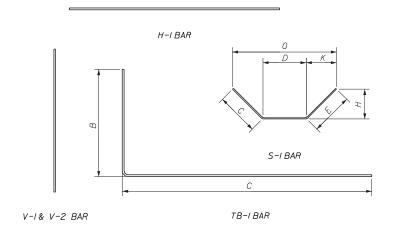
11643 103rd STREET JACKSONVILLE. FL 32210 PH: 1904) 778-2990 FX: 1904) 778-2992

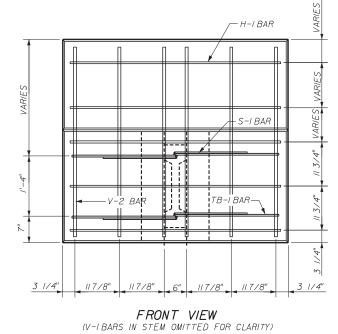
RETAINING WALL SYSTEM

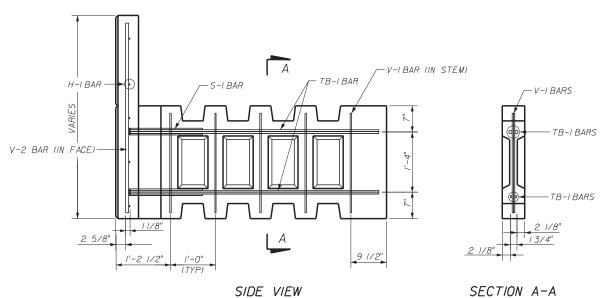
THE NEEL COMPANY T-WALL (2" COVER)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

	Names	Dates	Approve	d By /) 7	12/
Designed By	J MC	10/01/98	St	ate Structures D	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	15 of 21	l <i>5011</i> l





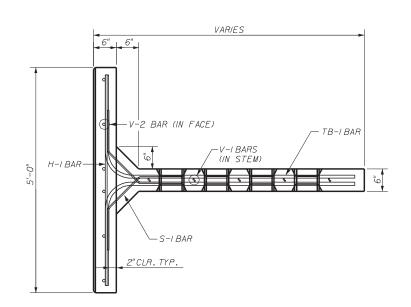


REBAR SCH	REBAR SCHEDULE - 5.5 x 5.0 x 08 TOP UNIT														
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS		
H-I	6	4	-	4'-6"								-			
V-/	14	3	-	2'-0"								-			
V-2	6	6	-	5'-0"								-			
S-/	4	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 %"	2'-13/4"	45			
TB-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90			

REBAR SCH	EDULE - 6.	0 x 5.0 x 0	8 TOP UNIT										
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	5'-6"								-	
5-/	4	3	3	2'-9"		9"	1'-3"	9"	5 %"	5 ¾"	2'-13/4"	4 5	
TR-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	

REBAR SCH	HEDULE - 6	5 x 5.0 x 0	8 TOP UNIT										
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	E	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	6'-0"								-	
S-/	4	3	3	2'-9"		9"	1'-3"	9"	5 %"	5 %"	2'-13/4"	45	
TR-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	

REBAR SCH	HEDULE - 7.	0 x 5.0 x 0	8 TOP UNIT										
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	14	3	-	2'-0"								-	
V-2	6	6	-	6'-6"								-	
S-/	4	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 %"	2'-13/4"	45	
TB-I	4	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	



TOP VIEW

REINFORCING STEEL -TOP UNITS (II)

THESE UNITS WILL ONLY BE USED BY APPROVAL OF THE F.D.O.T.STRUCTURES DESIGN OFFICE ON A PROJECT BY PROJECT BASIS.

DESIGNER:



ANY OL

PRECASTER:

OLDCASTLE PRECAST, INC

11643 103rd \$1REE1

JACKSOWILLE, FL 32210

PR: 19041 778-2990

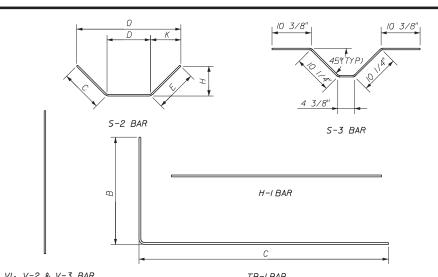
FX: 19041 778-2992

NOTE: ALL STEEL REINFORCING BARS SH

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

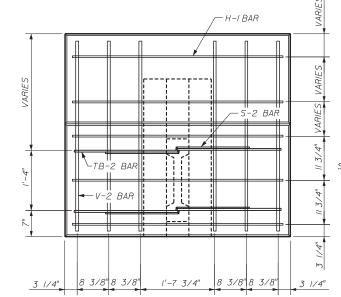
RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

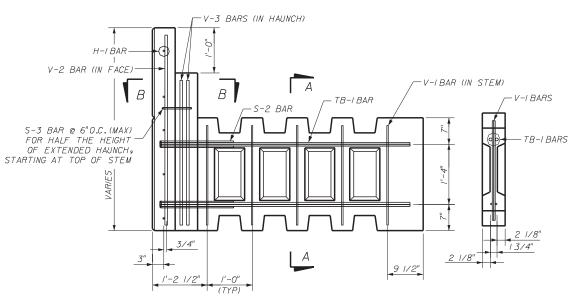
	Names	Dates	Approve	d By /) 7	12/
esigned By	J MC	10/01/98	St	ate Structures E	Design Engineer
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
Checked By	J MC	10/01/98	00	16 of 21	l <i>5011</i>



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VI,	V-2 8	· V-3	BAR					TB	−I BAR				
REBAR	SCHE DU L	E - 7.5	x 5.0 x	: 10 TOP UN	IT								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	8	4	_	4'-6"								-	
V-/	18	3	-	2'-0"								-	
V-2	6	6	-	7'-0"								-	
V-3	4	6	-	6'-0"								-	
S-2	4	3	3	2'-10"		10 %"	1'-2 3%"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
S-3	8	3	3	3'-3 %"	-1 - 1/1	-1 - 74 0						-	SEE BENDING DTL
TB-I	4	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 1/8"						90	
REBAR	SCHE DUL	E - 8.0	x 5.0 x	: 10 TOP UN	IT								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	18	3	-	2'-0"								-	
V-2	6	6	-	7'-0"								-	
V-3	4	6	-	6'-6"								-	
5-2	4	3	3	2'-10"		10 %"	1'-2 3/8"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
5-3	9	3	3	3'-3 %"								-	SEE BENDING DTL
TB-I	4	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 %"						90	
REBAR	SCHE DUL	E - 8.5	x 5.0 x	IO TOP UN	IT								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	18	3	-	2'-0"								-	
V-2	6	6	-	7'-0"								-	
V-3	4	6	-	7'-0"								-	
5-2	4	3	3	2'-10"		10 %"	1'-2 %"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
5-3	10	3	3	3'-3 %"								-	SEE BENDING DTL
TB-I	4	7	17	11'-7 1/8"	2'-2 1/4"	9'-6 %"						90	
DEDAD	SCHE DUL	E - 01	v 6.0 v	12 TOP UN	IT.						•		
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"	_ b	L L			"		T .	- ANOLL	NEMANNS
V-/	22	3	-	2'-0"								_	
V-2	6	6	-	7'-0"								_	
V-3	4	6	-	7'-6"								_	
5-2	4	3	3	2'-10"		10 %"	1'-2 %"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
S-3	//	3	3	3'-3 %"		10 /8	1 2 /8	10 /8	' 12	1 /2	12 3	-	SEE BENDING DTL
TB-I	4	7	17		2'-2 1/4"	II'−6 7/8"						90	SEE BENDING DIE
						70							
MARK	SCHE DUL			LGTH		_	_	-					
	QNTY	SIZE	TYPE		В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	8	4	_	4'-6"								-	
V-/	22	3	-	2'-0" 7'-0"								-	
V-2	6	6	-	8'-0"								-	
V-3	4	6				10 3/1	# O 3/#	10 3/11	7 1/2"	7 1/ *	2'-5"		
S-2 S-3	12	3	3	2'-10" 3'-3 %"		10 %"	1'-2 %"	10 %"	1 72	7 1/2"	2-5	45	SEE BENDING DTL
TB-I	12	7	17		2'-2 1/4"	//'-6 ½"		-			 	90	JLE DENDING DIL
					•	" U /B	l		l .				l
	SCHE DUL			x I2 TOP UI									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	22	3	-	2'-0"							-	-	
V-2	6	6	-	7'-0"							1	-	
V-3	4	6		8'-6"		***		.a. 7/	- ***	- // -	n/	-	
S-2	4	3	3	2'-10"		10 %"	1'-2 3/8"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
5-3	/3	3	3	3'-3 %"	01.0 1/1	u c 7/ =					-	-	SEE BENDING DTL
TB-I	4	7	17	13'-7 1/8"	•	//−6 ½"						90	
	SCHE DUL			x I2 TOP UI									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-/	22	3	-	2'-0"								-	
V-2	6	6	-	7'-0"								-	
V-3	4	6	-	9'-0"								-	
5-2	4	3	3	2'-10"		10 %"	1'-2 3/8"	10 %"	7 1/2"	7 1/2"	2'-5"	45	
5-3	14	3	3	3'-3 %"								-	SEE BENDING DTL
TB-I	4	7	17	13'-7 1/8"	2'-2 1/4"	11-6 1/8"						90	

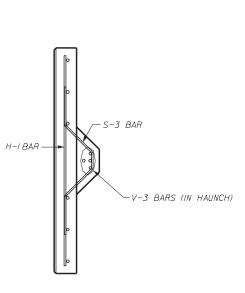




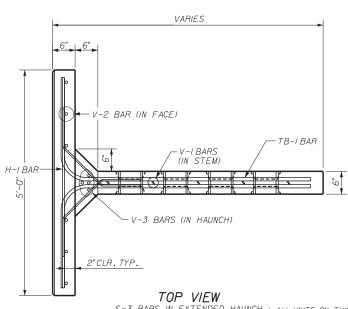
FRONT VIEW (V-IBARS IN STEM AND V-3 BARS IN HAUNCH OMITTED FOR CLARITY)

SIDE VIEW

SECTION A-A



SECTION B-B



S-3 BARS IN EXTENDED HAUNCH I. ALL UNITS ON THIS SHEET WILL ONLY BE USED BY OMITTED FOR CLARITY APPROVAL OF THE F.D.O.T. STRUCTURES DESIGN OFFICE ON A PROJECT BY PROJECT BASIS.

REINFORCING STEEL - TOP UNITS (III)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2"COVER)

	Names	Dates	Approve	d By /) 🦪	12/1
Designed By	J MC	10/01/98	S+	ate Structures E	esian Engineer
Drawn By	CAA	10/01/98		Sheet No.	Index No.
Checked By	J MC	10/01/98	00	17 of 21	5011

DESIGNER:



THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
Ptk: 17031 913-7858
Fx: 17031 913-7858

PRECASTER:

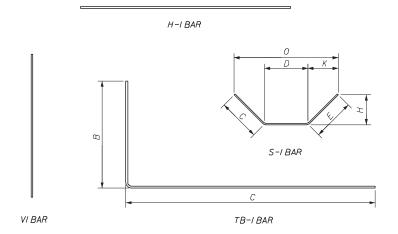
OLDCASTLE PRECAST, INC

11643 10346 STREET

JACKSONVILLE, FI 32210

Pr. 69041 778-29992

Fr. 69041 778-29992



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REBAR S	CHEDULE	- 5.0	x 5.0 .	x 04 DBL UN	I/T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-/	12	3	-	4'-6"								-	
5-1	8	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	4	17	5'-8 1/2"	2'-3 1/2"	3'-6 1/2"						90	
REBAR S	CHE DU LE	- 5.0	x 5.0	x 06 DBL UN	IIT			•				•	
REBAR S	CHEDULE QNTY	- 5.0 SIZE	x 5.0 .	x 06 DBL UN LGTH	IIT B	С	D	E	Н	К	0	ANGLE	REMARKS
						С	D	Ε	Н	К	0	ANGLE -	REMARKS
MARK	QNTY	SIZE	TYPE	LGTH		С	D	E	Н	К	0		REMARKS
MARK H-I	QNTY 6	SIZE	TYPE -	LGTH 4'-6"		C	D	E	H 8"	K 8"	0 2' -3 3/4"	-	REMARKS

18-1	8	4	- 17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90		
REBAR S	SCHE DU LE	- 5.0	x 5.0	x 08 DBL UI	V/T									-
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS	
H-I	6	4	-	4'-6"								-		
V-/	20	3	-	4'-6"								-		
S-I	8	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45		
TB-I	8	4	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90		

REBAR S	CHEDULE	- 5.0	x 5.0	x IO DBL UN	ΙΤ								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-/	24	3	-	4'-6"								-	
5-1	8	3	3	2'-9 %"		11 1/4"	11 1/4"	111/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	4	17	11'-8 1/2"	2'-3 1/2"	9'-6 1/2"						90	

REBAR S	CHEDULE	- 5.0	x 5.0	x 12 DBL UN	11T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-/	6	4	-	4'-6"								-	
V-/	26	3	-	4'-6"								-	
S-I	8	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	4	17	13'-8 1/2"	2'-3 1/2"	11'-6 1/2"						90	

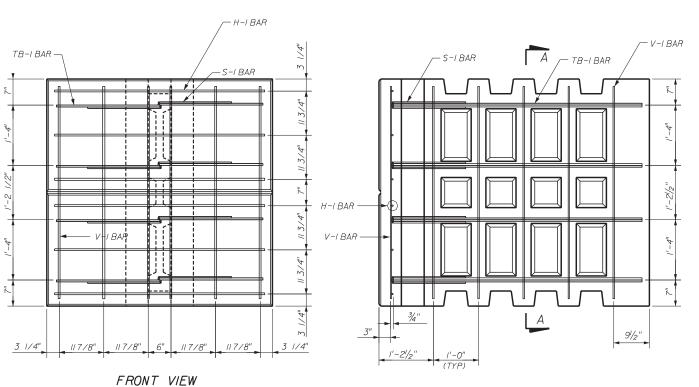
REBAR S	CHEDULE	- 5.0	x 5.0	x 14 DBL UN	IT								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-/	6	4	-	4'-6"								-	
V-/	32	3	-	4'-6"								-	
S-I	8	3	3	2'-9 %"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	4	17	15'-8 1/2"	2'-3 1/2"	15'-6 1/2"						90	

REBAR S	<i>CHEDULE</i>	- 5.0	x 5.0	x 16 DBL UN	IT.								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	K	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-/	36	3	-	4'-6"								-	
S-I	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	4	17	17'-8 1/2"	2'-3 1/2"	15'-6 1/2"						90	

DESIGNER:



THE NEEL COMPANY
8328-0 TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (703) 913-7858
FX: (703) 913-7859

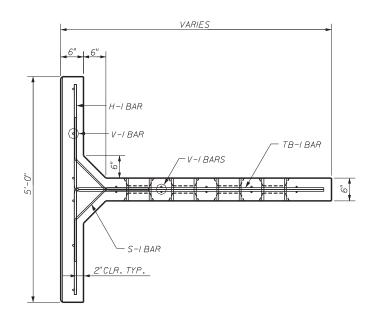


(V-IBARS IN STEM OMITTED FOR CLARITY)

- TB-I BARS

√ V−I BARS

SECTION A-A



SIDE VIEW

TOP VIEW REINFORCING STEEL -DOUBLE UNITS

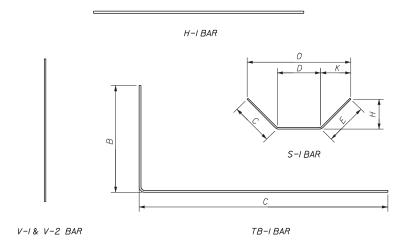
PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 1904 1778-2999
FX: 19041 778-2992

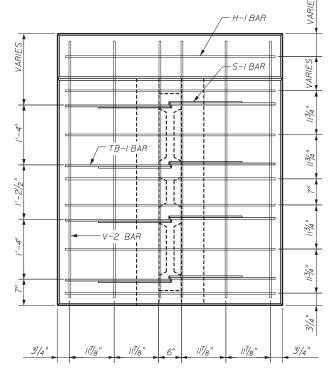
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approve	d By	V 21.T			
signed By	J MC	10/01/98	State Structures Design Engineer					
awn By	CAA	10/01/98	Revision	Sheet No.	Index No.			
ecked By	J MC	10/01/98	00	18 of 21	5011			



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FRONT VIEW
(V-I BARS IN STEM OMITTED FOR CLARITY)

H-I BAR -V-2 BAR-91/2" 1'-21/2"

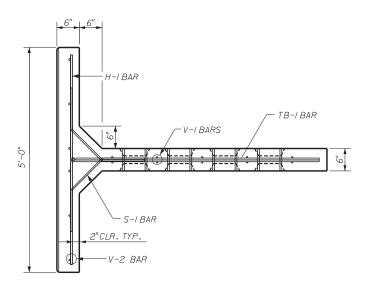
— TB-I BARS 13/4"

← V-I BARS

SIDE VIEW

SECTION A-A

				DBL TOP U									
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	E	Н	K	0	ANGLE	REMARKS
H-I	6	4	-	4'-6"								-	
V-I	10	3	-	4'-6"								-	
V-2	6	5	-	5'-0"								-	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
REBAR :	REBAR SCHEDULE - 6.0 x 5.0 x 06 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"								-	
V-I	10	3	-	4'-6"								-	
V-2	6	5	-	5'-6"								-	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 ¾"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
REBAR S	REBAR SCHEDULE - 6.5 x 5.0 x 06 DBL TOP UNIT												
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	E	Н	К	0	ANGLE	REMARKS
H-I	7	4	-	4'-6"							-	-	
V-I	10	3	-	4'-6"								-	
V-2	6	5	-	6'-0"								-	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	
DEDAR 1	CCHEDILLE	70.	. 50 . 00	DBL TOP UI	uit.								•
MARK	QNTY	- 7.0) SIZE	TYPE	LGTH	WII	С	D	E		V	0	ANGLE	REMARKS
H-I	8	31ZE 4	11PE	4'-6"	В	L	D	E	Н	K	U	ANGLE -	REMARNS
V-I	10	.3	_	4'-6"									
V-1 V-2	6	5	_	6'-6"								_	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 3/4"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	7.7	11 74	11.74	0	0	2-3-74	90	
16-1	0)	11	1 -0 72	2-5 /2	3-6 72						30	
REBAR :	SCHE DU LE	- 7.5	x 5.0 x 06	DBL TOP UI	VIT								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	E	Н	K	0	ANGLE	REMARKS
H-I	8	4	-	4'-6"								-	
V-I	10	3	-	4'-6"								-	
V-2	6	5	-	7'-0"								-	
S-/	8	3	3	2'-9 1/8"		11 1/4"	11 1/4"	11 1/4"	8"	8"	2'-3 ¾"	45	
TB-I	8	5	17	7'-8 1/2"	2'-3 1/2"	5'-6 1/2"						90	



TOP VIEW REINFORCING STEEL - DOUBLE TOP UNITS (I)

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 2" MIN. CONCRETE COVER

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approve	d By	12/			
igned By	J MC	10/01/98	State Structures Design Engineer					
wn By	CAA	10/01/98	Revision	Sheet No.	Index No.			
cked By	J MC	10/01/98	00	19 of 21	5011			

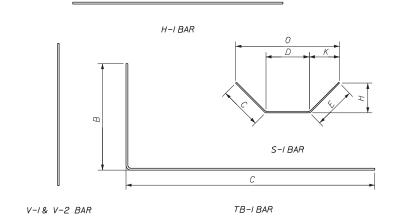
DESIGNER:



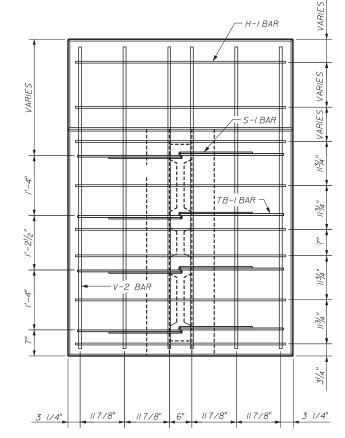
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17031 913-7858
FX: 17031 913-7859

PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 19041 778-2990
FX: 19041 778-2992



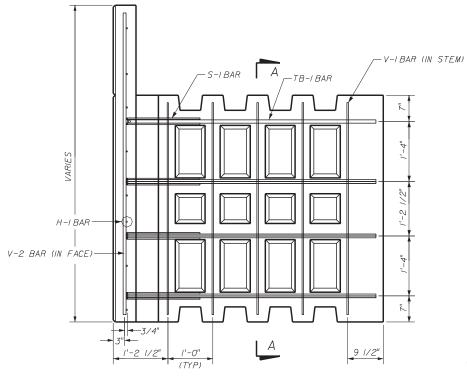
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FRONT VIEW

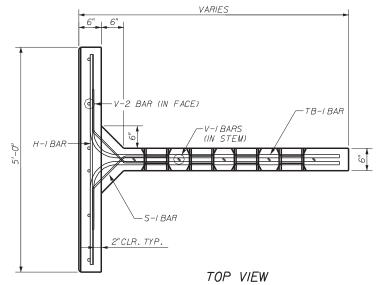
(V-1 BARS IN OMITTED FOR CLARITY)

REBAR S	SCHEDULE	- 8.0 x 5	.0 x 08 l	DBL TOP UNI	T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-/	9	4	-	4'-6"								-	
V-/	14	3	-	4'-6"								-	
V-2	6	6	-	7'-6"								-	
5-1	8	3	3	2'-9"		9"	/'-3"	9"	5 %"	5 %"	2'-134"	45	
TB-I	8	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	
REBAR S	SCHEDULE	- 8.5 x 5	5.0 x 08 i	DBL TOP UNI	'T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	9	4	-	4'-6"								-	
V-/	14	3	-	4'-6"								-	
V-2	6	6	-	8'-0"								-	
5-1	8	3	3	2'-9"		9"	/'-3"	9"	5 ¾"	5 ¾"	2'-13/4"	45	
TB-I	8	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	
REBAR S	SCHEDULE	- 9.0 x 5	i.0 x 08 l	DBL TOP UNI	T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	10	4	-	4'-6"								-	
V-/	14	3	-	4'-6"								-	
V-2	6	6	-	8'-6"								-	
5-1	8	3	3	2'-9"		9"	/'-3"	9"	5 3/8"	5 %"	2'-13/4"	45	
TB-I	8	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"						90	
RFRAR S	SCHEDULE	- 9.5 x 5	5.0 x 08 I	DBL TOP UNI	'T								
MARK	QNTY	SIZE	TYPE	LGTH	В	С	D	Ε	Н	К	0	ANGLE	REMARKS
H-I	10	4	-	4'-6"		-	_					-	
V-/	14	3	-	4'-6"								-	
V-2	6	6	-	9'-0"								_	
5-1	8	3	3	2'-9"		9"	/'-3"	9"	5 3/8"	5 3/8"	2'-13/4"	45	
TB-I	8	6	17	9'-8 1/2"	2'-3 1/2"	7'-6 1/2"				- 70	+	90	t



SIDE VIEW

SECTION A-A



REINFORCING STEEL - DOUBLE TOP UNITS (II)

THESE TWO UNITS WILL ONLY BE USED BY — APPROVAL OF THE F.D.O.T. STRUCTURES DESIGN OFFICE ON A PROJECT BY PROJECT BASIS.

DESIGNER:



THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031 913-7858
FX: (7031 913-7859

PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FL 32210
PH: 19041 1778-2990
FX: 19041 778-2992

NOTE: ALL STEEL REINFORCING BARS SHALL HAVE 2" MIN. CONCRETE COVER

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

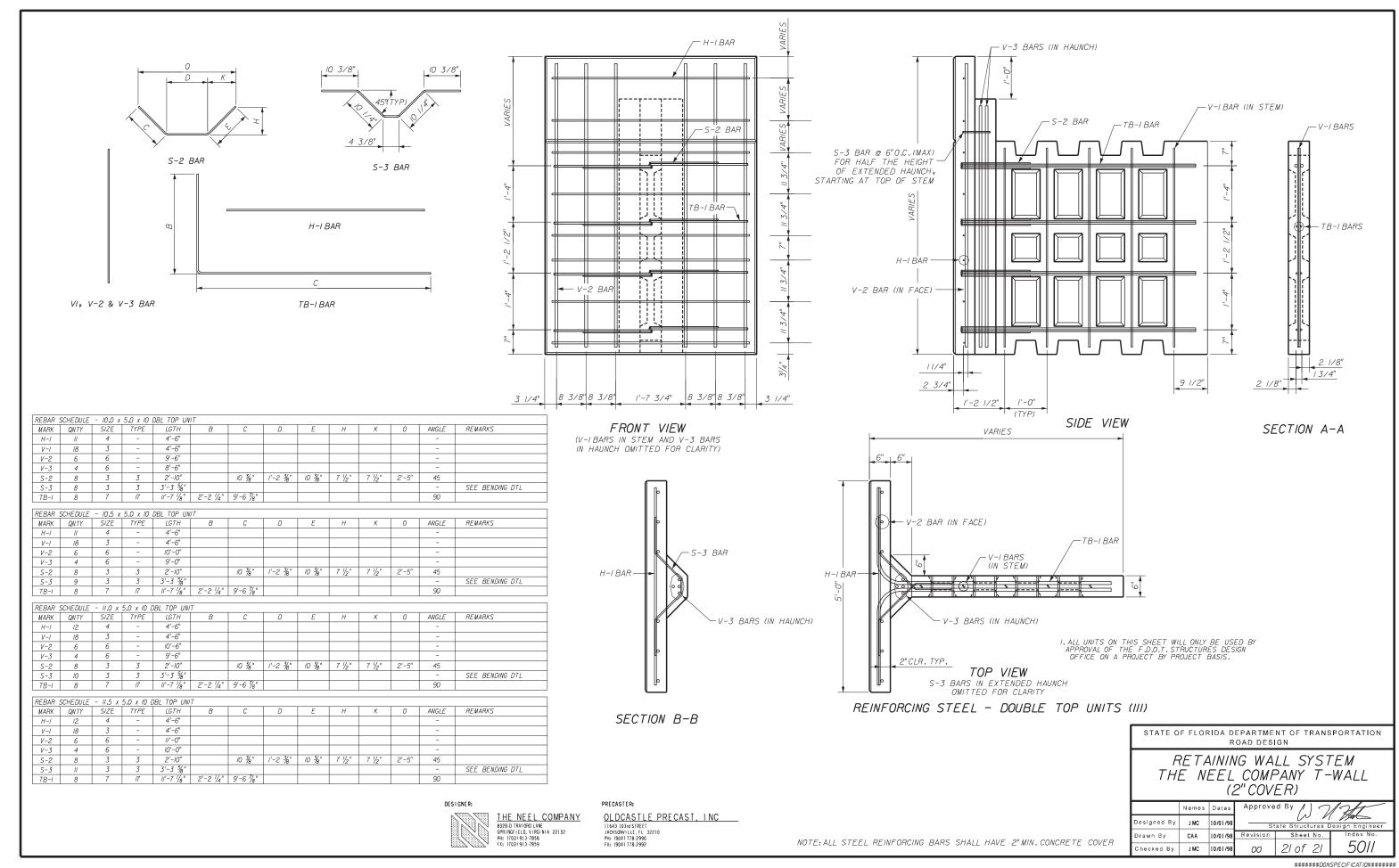
__ V-I BARS

-13/4"

— TB-I BARS

RETAINING WALL SYSTEM THE NEEL COMPANY T-WALL (2" COVER)

	Names	Dates	Approve	d By /) ?	12/1
signed By	J MC	10/10/98	St	ate Structures E	Design Engineer
awn By	CAA	10/10/98	Revision	Sheet No.	Index No.
necked By	J MC	10/10/98	00	20 of 21	5011



STANDARD DETAILS

ISOGRID® M.S.E. WALL SYSTEM

DESIGNER



THE NEEL COMPANY

8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER

OLDCASTLE PRECAST, INC.

II643 IO3rd STREET JACKSONVILLE, FL 322IO PH: (904) 778-2990 FX: (904) 778-2992

LEGEND



PANEL WITH ONE SOIL REINFORCEMENT GRID



HORIZONTAL HALF-PANEL WITH ONE SOIL REINFORCEMENT GRID



PANEL WITH TWO SOIL REINFORCEMENT GRIDS



VERTICAL HALF-PANEL WITH ONE SOIL REINFORCEMENT GRID



PANEL WITH THREE SOIL REINFORCEMENT GRIDS



TL/BR & TR/BL QUARTER PANELS WITH ONE SOIL REINFORCEMENT GRID



PANEL WITH FOUR SOIL REINFORCEMENT GRIDS



SPECIAL HEIGHT PANELS (X-ITHRU X-5)WITH ONE SOIL REINFORCEMENT GRID

DESIGNER:



THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER:

OLDCASTLE PRECAST, INC

11643 103rd STREET JACKSONVILLE, FL 32210 PH: 19041 778-2990 FX: 17031 913-7859

THIS SYSTEM SHALL BE USED IN MODERATELY OR SLIGHTLY AGGRESSIVE ENVIRONMENTS ONLY

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM THE NEEL COMPANY ISOGRID

Names Dates Designed By J MC 10/01/98 CAA | 10/01/98 hecked By JMC 10/01/98 00 1 of 20

\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$

MISCELLANEOUS NOTES:

I. DESIGNER:

THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VA 22/52 PH: (703) 9/3-7858 FX: (703) 9/3-7859

2. PRECASTER:

OLDCASTLE PRECAST INC. 11643 103rd STREET JACKSONVILLE, FL 32210 PH: (904) 778-2990 FX: (904) 778-2992

3. MATERIALS SUPPLIED BY PRECASTER:

-PRECAST ISOGRID PANELS

-GALVANIZED SOIL REINFORCEMENT GRID

-GALVANIZED GRID LOCKING BAR

-DIAGONAL JOINT MATERIAL AND ADHESIVE

-VERTICAL JOINT MATERIAL

DESIGN NOTES:

I. DESIGN IS BASED ON THE ASSUMPTION THAT
THE MATERIAL WITHIN THE RETAINING WALL
VOLUME, METHODS OF CONSTRUCTION, AND
QUALITY OF PREFABRICATED MATERIALS SHALL
CONFORM TO SPEC SECTION 548 - RETAINING
WALL SYSTEMS.

2. SOIL PARAMETERS:

-SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS
OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE
WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN
PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL
SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUE OF 0,
C AND GAMMA SHALL BE PROVIDED IN THE SHOP DRAWINGS

3. FACTORS OF SAFETY:

-OVERTURNING - 2.0

-SLIDING - 1.5

-INTERNAL PULLOUT - 1.5 (ALLOWABLE DEFORMATION 0.75")
-SOIL REINFORCEMENT GRID - 0.47 Fy AT END OF DESIGN LIFE

-BEARING CAPACITY - 2.5

-OVERALL STABILITY - 1.5

- 4. THE MAXIMUM APPLIED BEARING PRESSURE AT THE FOUNDATION LEVEL IS AS SHOWN ON THE WALL DESIGN DRAWINGS FOR EACH DESIGN CASE. IT IS THE RESPONSIBILITY OF THE OWNER TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 5. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY THE OWNER. ON THE BASIS OF THIS INFORMATION, THE NEEL COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY, IS THE RESPONSIBILTY OF OTHERS

MATERIALS NOTES:

I. PRECAST CONCRETE:

-PRECAST ISOGRID PANELS - PER SPEC SECTION 548
-ARCHITECTURAL FINISH SHALL BE PLAIN STEEL FORM FINISH
UNLESS OTHERWISE SPECIFIED ON THE RETAINING WALL CONTROL PLANS.

2. C.I.P. CONCRETE:

-C.I.P. LEVELING PAD - PER SPEC SECTION 548 -OTHER C.I.P. CONCRETE - PER SPEC SECTION 548

3. REINFORCING STEEL:

-PER SPEC SECTION 548

-6" x 6" WELDED GRID, D8 x D8 WIRE

OR

-#3 REBAR @ 6"O.C. EACH WAY -WELDED PER ASTM A497 PRIOR TO GALVANIZATION

4. CONNECTION INSERT:

-PER SPEC SECTION 548

-WII WIRE

-WELDED PER ASTM A185 PRIOR TO GALVANIZATION

5. LOCKING BAR:

-PER SPEC SECTION 548

6. SOIL REINFORCEMENT GRIDS:

-PER SPEC SECTION 548

-WII WELDED WIRE GRIDS:

-5 LONGITUDINAL WIRES @ 6"O.C., LENGTH AS REQUIRED BY DESIGN
-2'LONG TRANSVERSE BARS AT 6"OR 12"O.C., AS REQUIRED BY DESIGN
-SOIL GRID LENGTHS SHOWN ON ISOGRID DESIGN DRAWINGS ARE NOMINAL
LENGTHS AS REQUIRED BY DESIGN CALCULATIONS. DUE TO MANUFACTURING
TOLERANCES, ACTUAL GRID LENGTHS MAY BE LONGER.

AND COMPACTED BEFORE WALL CONSTRUCT
20'IN HEIGHT.

20'IN HEIGHT.

7. IF EXISTING OR FUTURE STRUCTURES, PIPES,
FOUNDATIONS OR GUARDRAIL POSTS WHICH A

7. JOINT MATERIAL:

-DIAGONAL JOINT FILLER:

-1/2" x 4" x 4'-2"

-PREFORMED EPDM

-DUROMETER: 80 - 90

-DIAGONAL JOINT BACKING:

-MIRAFI 140N OR EQUAL

-12"WIDE x LENGTH OF JOINT

-GEOTEXTILE MEETING REQUIREMENTS OF SPEC SECTION 548

-WEEPHOLE COVER:

-TENSAR DC4205 OR EQUAL

-6" x 61/2" (MIN)

-GEOCOMPOSITE MEETING REQUIREMENTS OF SPEC SECTION 548

8. BACKFILL:

-PER SPEC SECTION 548

CONSTRUCTION NOTES:

- I. ALL CONSTRUCTION PROCEDURES SHALL COMPLY
 WITH SPEC SECTION 548-8 AND THE "ISOGRID
 CONSTRUCTION MANUAL" (PROVIDED BY THE NEEL
 COMPANY OR OLDCASTLE PRECAST, INC. IN THE
 EVENT OF A DISCREPANCY BETWEEN THE SPEC AND
 THE "ISOGRID CONSTRUCTION MANUAL", THE SPEC
 SHALL CONTROL.
- 2. FOR LOCATION AND ALIGNMENT OF ISOGRID STRUCTURE, SEE RETAINING WALL CONTROL PLANS.
- 3. ISOGRID STRUCTURES ON CURVES SHALL BE BUILT IN CHORDS AS SHOWN IN THE ISOGRID DESIGN DRAWINGS.
- 4. IF MANHOLES OR DROP INLETS ARE PRESENT, THEY
 SHALL BE LOCATED AS SHOWN IN THE ISOGRID DESIGN
 DRAWINGS.
- 5. IF PILES ARE LOCATED WITHIN THE RETAINING WALL
 VOLUME, THEY SHALL BE DRIVEN BEFORE CONSTRUCTION
 OF THE ISOGRID STRUCTURE UNLESS A METHOD TO PROTECT
 THE STRUCTURE, WHICH IS ACCEPTABLE TO THE ENGINEER
 AND THE NEEL COMPANY, IS SUBMITTED AND APPROVED
 IN WRITING.
- 6. IF A STRUCTURE EXCEEDS 20'IN HEIGHT, THE FINISH
 GRADE AT THE FACE OF THE WALL SHALL BE PLACED
 AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS
 20'IN HEIGHT.
- 7. IF EXISTING OR FUTURE STRUCTURES, PIPES,
 FOUNDATIONS OR GUARDRAIL POSTS WHICH ARE WITHIN
 THE RETAINING WALL VOLUME INTEFERE WITH THE
 NORMAL PLACEMENT OF REINFORCING GRIDS AND
 SPECIFIC DIRECTION HAS NOT BEEN PROVIDED IN THE
 CONTRACT DOCUMENTS, THE CONTRACTOR SHALL NOTIFY
 THE FNGINFFR.
- 8. TOP PANELS ON WALLS WITH CAST-IN -PLACE COPING SHALL HAVE *4 REBAR PROTRUDING FROM THEIR TOP EDGE.
- 9. BACKFILL MATERIAL SHALL BE PLACED AND COMPACTED
 IN ACCORDANCE WITH THE SPECIFICATIONS FOR MSE
 WALLS TO A LEVEL OF APPROXIMATELY 2" ABOVE THE
 CONNECTION INSERT EMBEDDED IN THE PANELS.
 INSTALLATION OF THE SOIL REINFORCEMENT SHALL BE
 PERMITTED ONLY AFTER PLACEMENT AND COMPACTION OF
 THE BACKFILL MATERIAL HAS REACHED THE REQUIRED LEVEL.
- IO. COMPACTION AND OPERATION EQUIPMENT SHALL BE KEPT

 A MINIMUM DISTANCE OF 3' FROM THE BACK FACE OF THE ISOGRID PANELS. COMPACTION WITHIN 3' OF THE ISOGRID PANEL SHALL BE 90% OF AASHTO T-180.
- II. THE CONTRACTOR IS RESPONSIBLE FOR GRADUALLY

 DEFLECTING UPPER REINFORCING GRIDS DOWNWARD

 TO AVOID CONFLICTS WITH PAVING AND SUBGRADE

 PREPERATION. THE CONTRACTOR'S ATTENTION IS DIRECTED

 ESPECIALLY TO SITUATIONS WHERE ROADWAY

 SUPERELEVATION AND/OR SOIL MIXING ARE ANTICIPATED.
- I2. THE CONTRACTOR IS RESPONSIBLE FOR CONTROLLING STORM

 WATER DRAINAGE IN THE VICINITY OF THE WALL DURING

 CONSTRUCTION. STORMWATER RUNOFF SHALL BE COLLECTED

 AND DISCHARGED AWAY FROM THE WALL AND THE RETAINING

 WALL VOLUME.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

Names Dates Approved By JMC 10/01/98 State Roadway Design Engineer Sheeked By JMC 10/01/98 Revision Sheet No. Index No. Checked By JMC 10/01/98 00 2 of 20 50/2

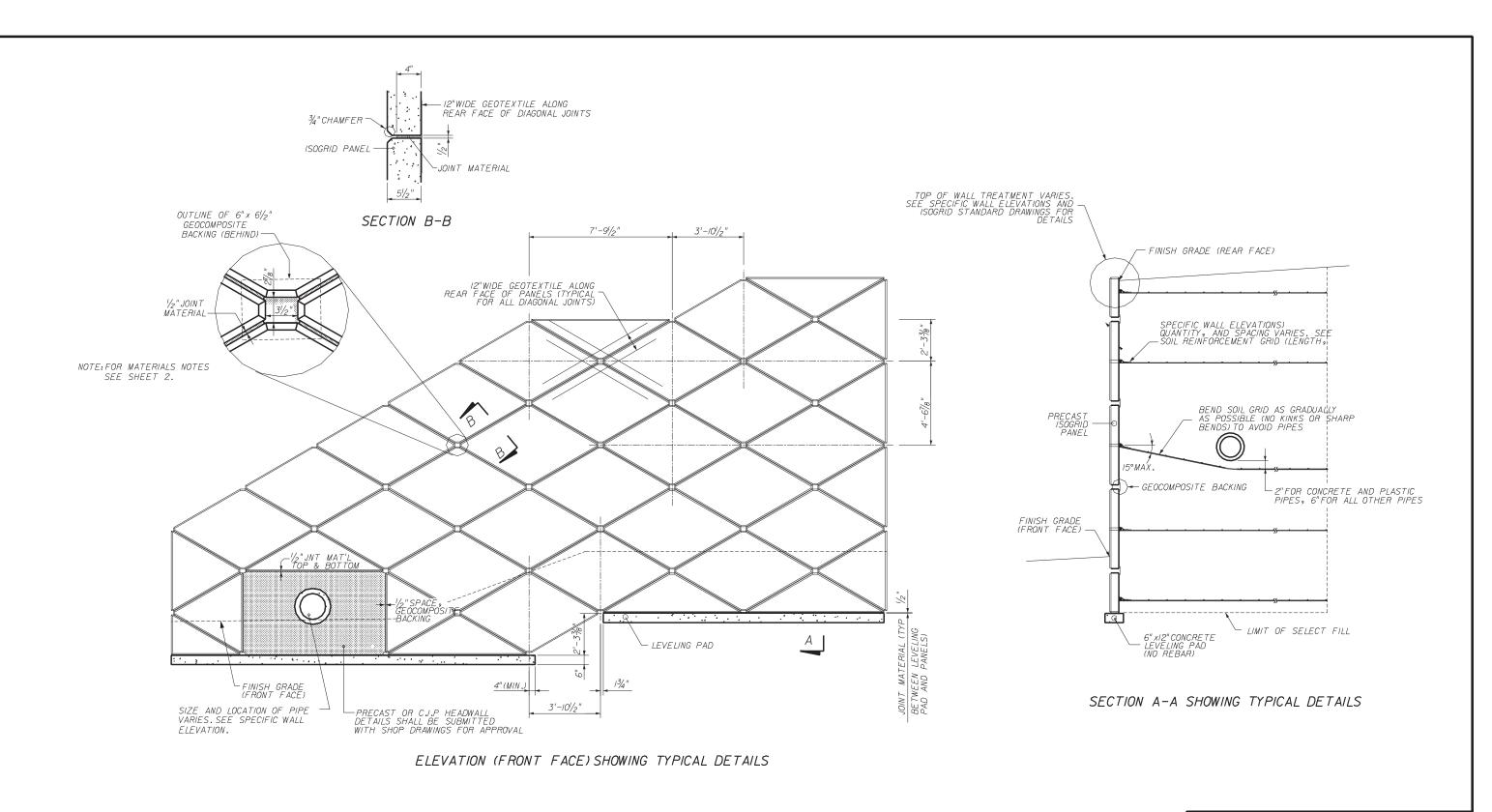
DESIGNER:

THE NEEL COMPANY
8228-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17031 913-7858

PRECASTER:

OLDCASTLE PRECAST, INC

11643 103rd STREET
JACKSONVILLE, FL 32210
PR: (904) 778-2990
FX: (703) 913-7859



STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

DESIGNER:

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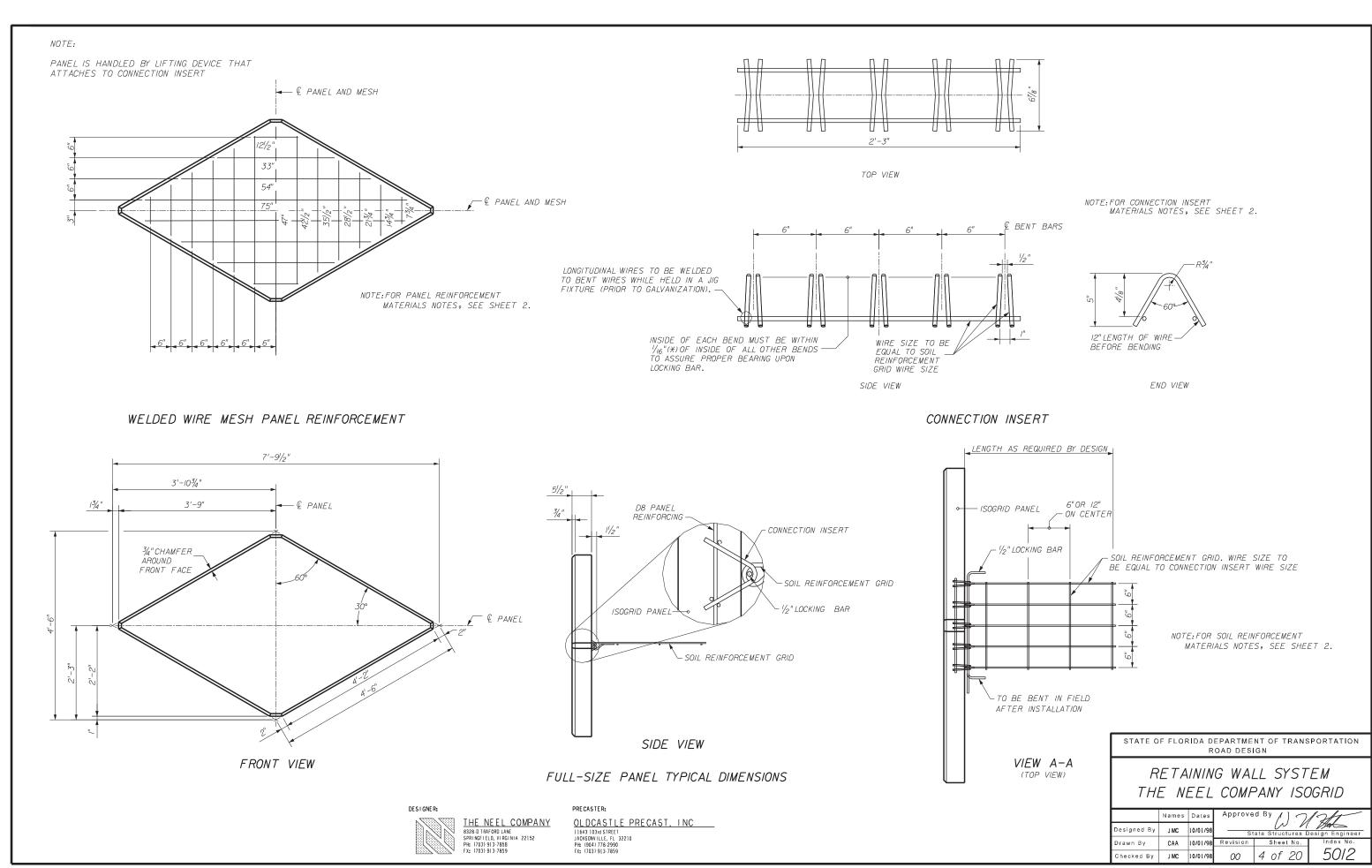


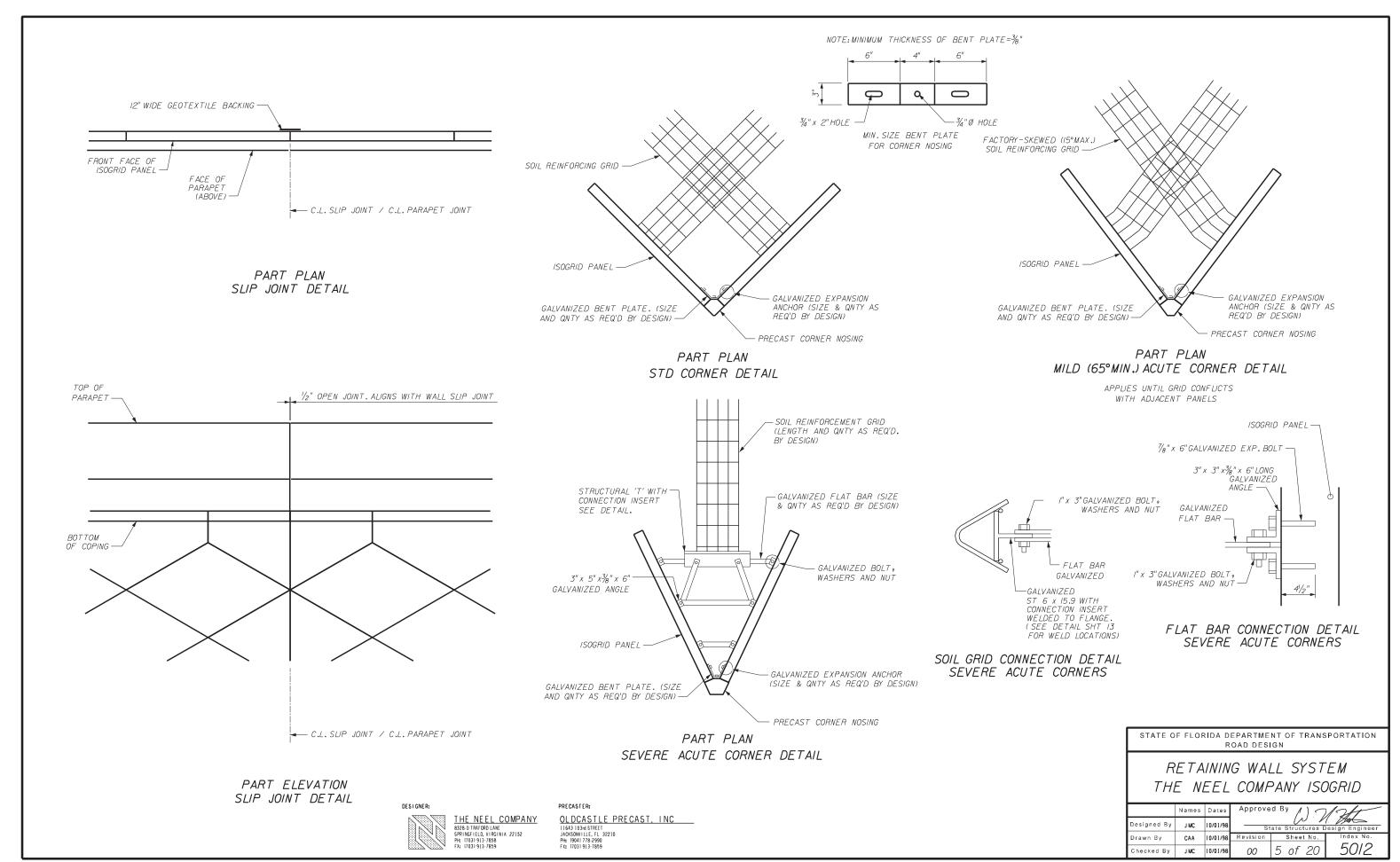
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17031 913-7858
FX: 17031 913-7859

PRECASTER:

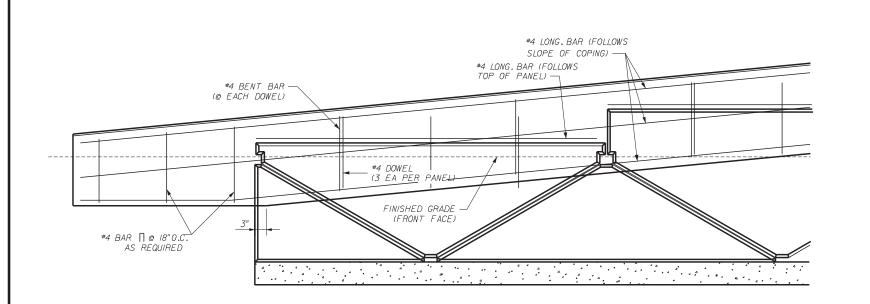
OLDCASTLE PRECAST, INC

11643 10346 STREET
JACKSOWYLLE, FL 32210
PR: (704) 378-2990
FX: (703) 913-7859



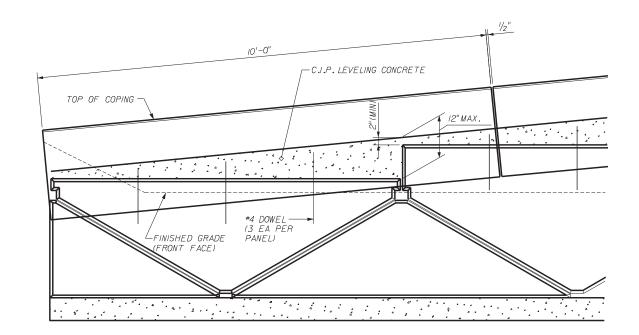


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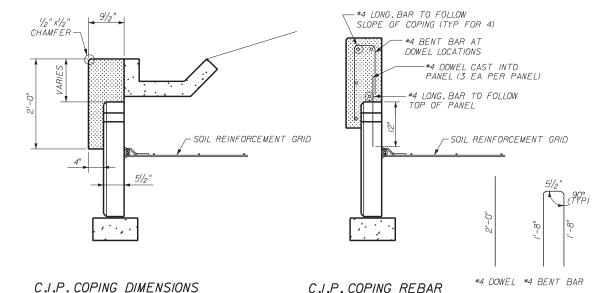


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C.I.P. COPING TREATMENT AT BEGINNING/END OF WALLS

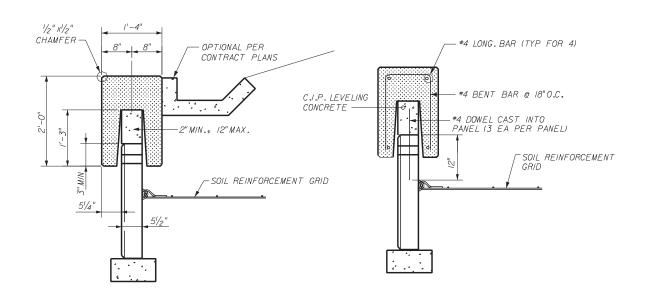


PRECAST COPING - PART ELEVATION



C.I.P. COPING REBAR DETAILS BENT BAR AND DOWEL

TO BE FIELD-TRIMMED
AS REQUIRED TO
PROVIDE MIN OF 2" OF CONCRETE COVER



PRECAST COPING DIMENSIONS

PRECAST COPING REBAR

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE NEEL COMPANY ISOGRID

	Names	Dates	Approved By				
signed By	J MC	10/01/98	State Structures Design Engineer				
awn By	CAA	10/01/98	Revision	Sheet No.	Index No.		
ecked By	J MC	10/01/98	00	6 of 20	5012		

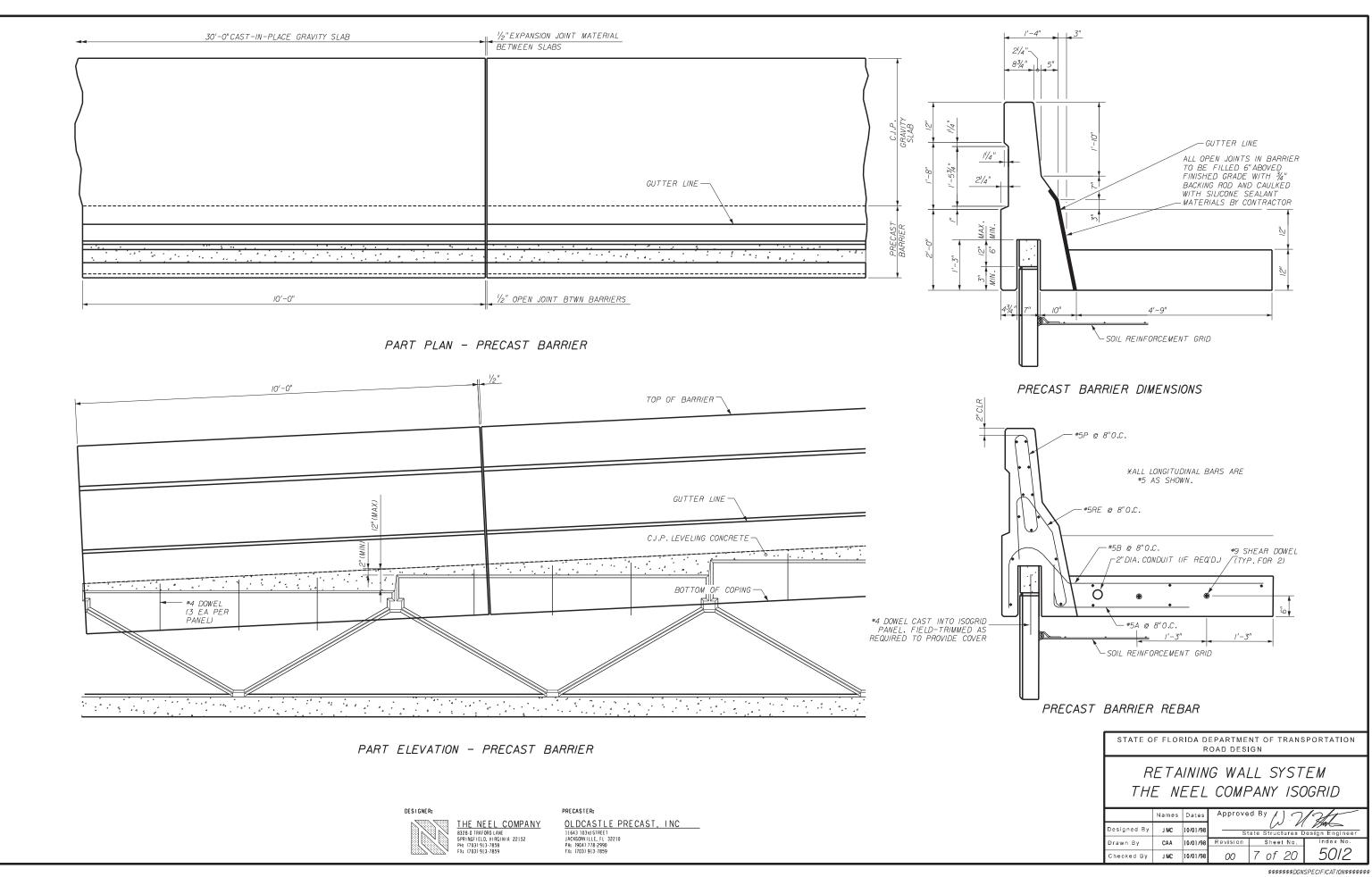
DESIGNER:



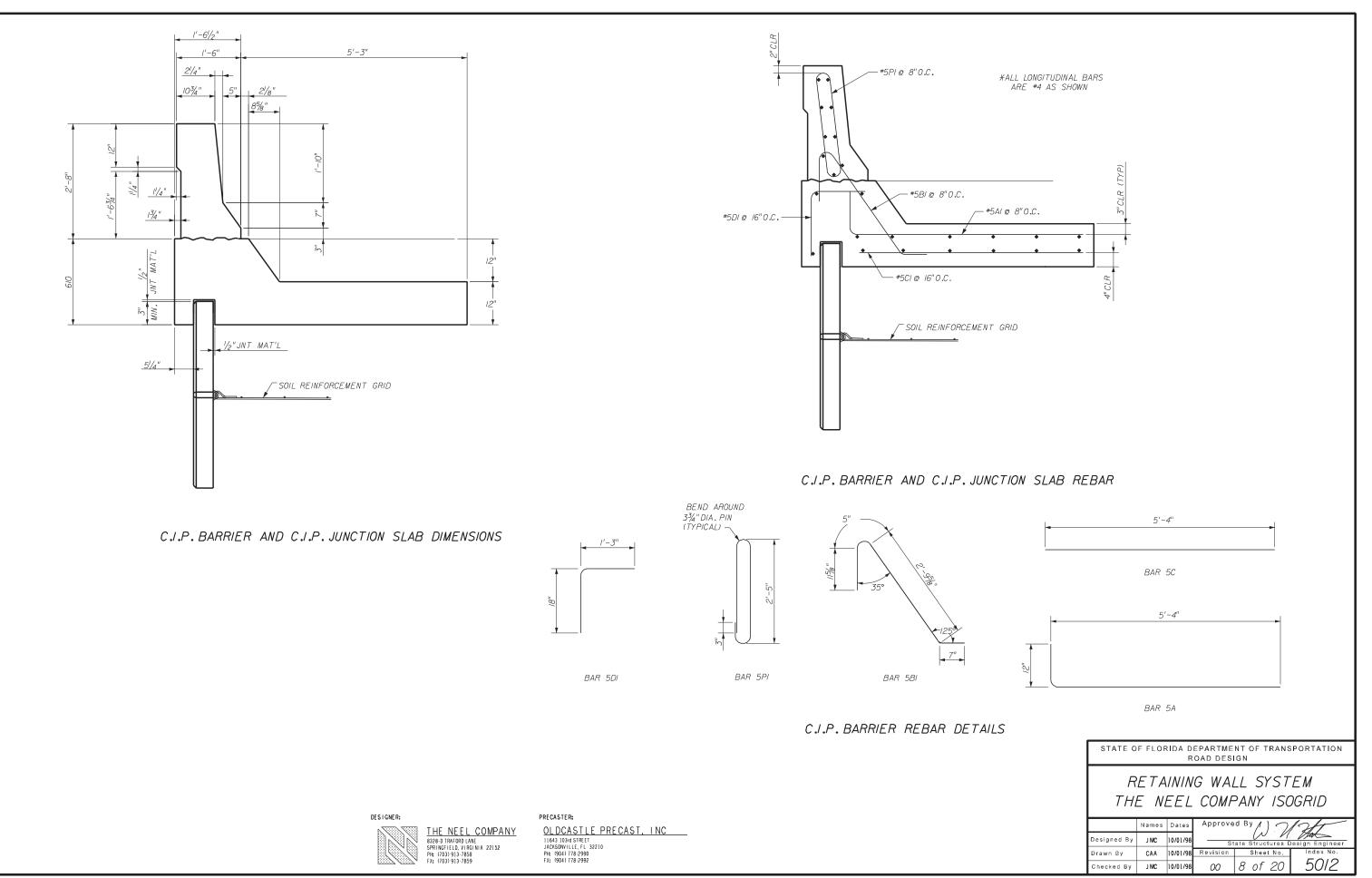
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 1703/913-7858
FX: 1703/913-7859

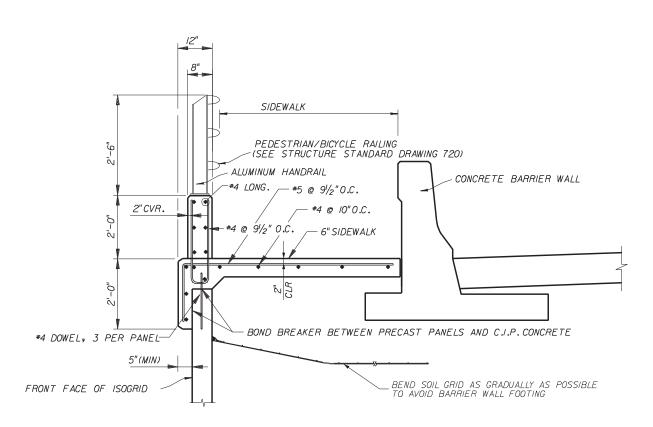
OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSOWILLE, FL 32210
PR: (904) 778-2990
FX: (703) 913-7859

PRECASTER:



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C.I.P. PARAPET DETAIL W/ HANDRAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

Designed By JMC 10/01/98 State Structures Design Engineer

Drawn By CAA 10/01/98 Revision Sheet No. Index No.

Checked By JMC 10/01/98 00 9 of 20 50/2

DESIGNER:

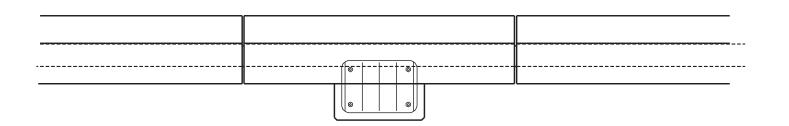
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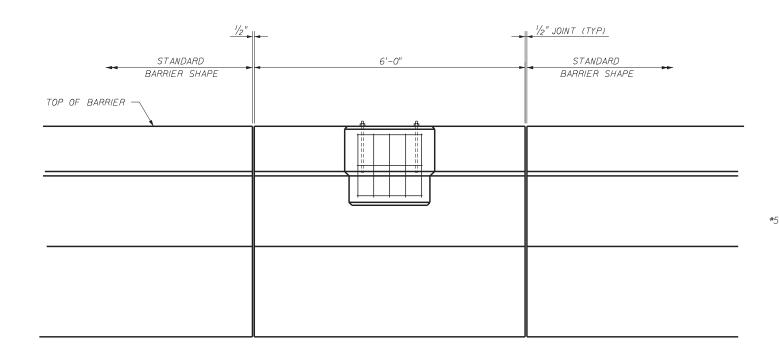
THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031 913-7858
FX: (7031 913-7859

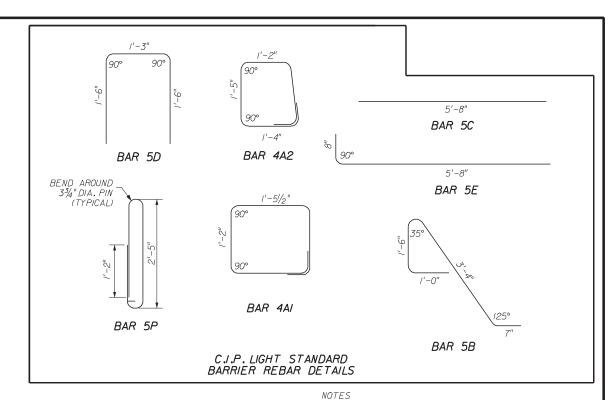
OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, FI 32210
PH: 19041 1778-2990
FX: 19041 778-2992

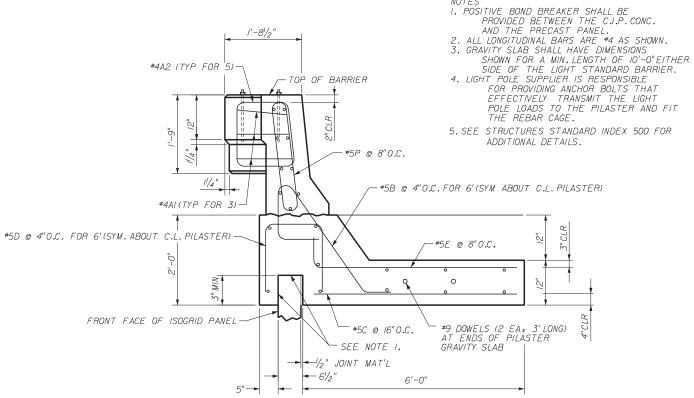
PRECASTER:



C.J.P. LIGHT STANDARD BARRIER - PART PLAN WITH REBAR (BARRIER AND GRAVITY SLAB REBAR OMITTED FOR CLARITY)







C.I.P. LIGHT STANDARD BARRIER - PART SECTION WITH REBAR

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

C.J.P. LIGHT STANDARD BARRIER - PART ELEVATION (BARRIER AND GRAVITY SLAB REBAR OMITTED FOR CLARITY)

DESIGNER:

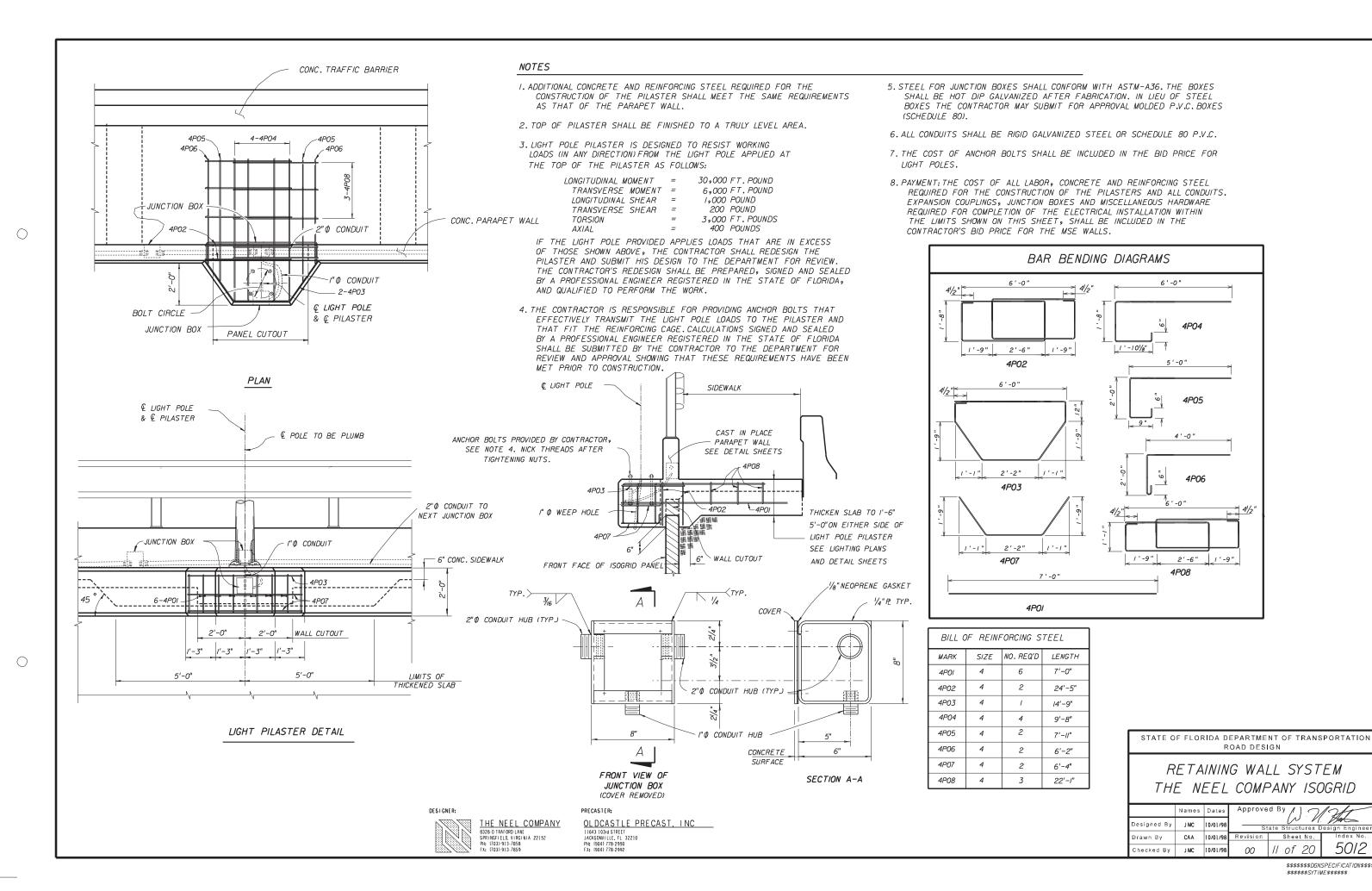


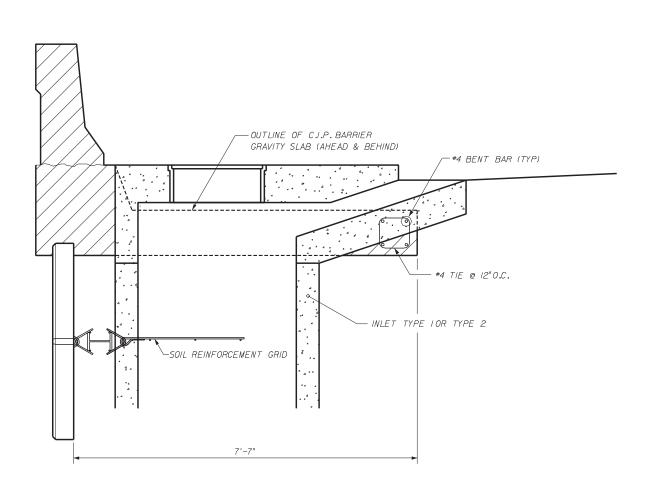
THE NEEL COMPANY
8328-0 TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031913-7858
FX: (7031913-7859

PRECASTER:

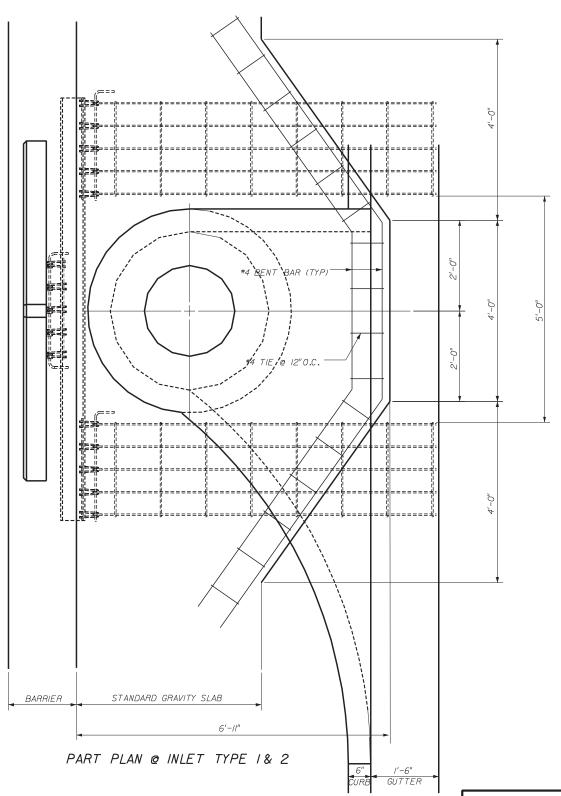
OLDCASTLE PRECAST, INC

11643 103rd STREET
JACKSOWI LIE, FL 32210
PRE 19941 7782-990





PART SECTION @ INLET TYPE 1 & 2
NOTE: GRID RELOCATION HARDWARE, SEE SHEET 13 OF 20 FOR DETAILS



GRAVITY SLAB AT INLET TYPE 1 & 2

DESIGNER:

THE NEEL COMPANY
8328-D TRAFORD LANE
SPRING FILD, VIGINIA 22152
PH. (703191-7859
Fit. (7031913-7859

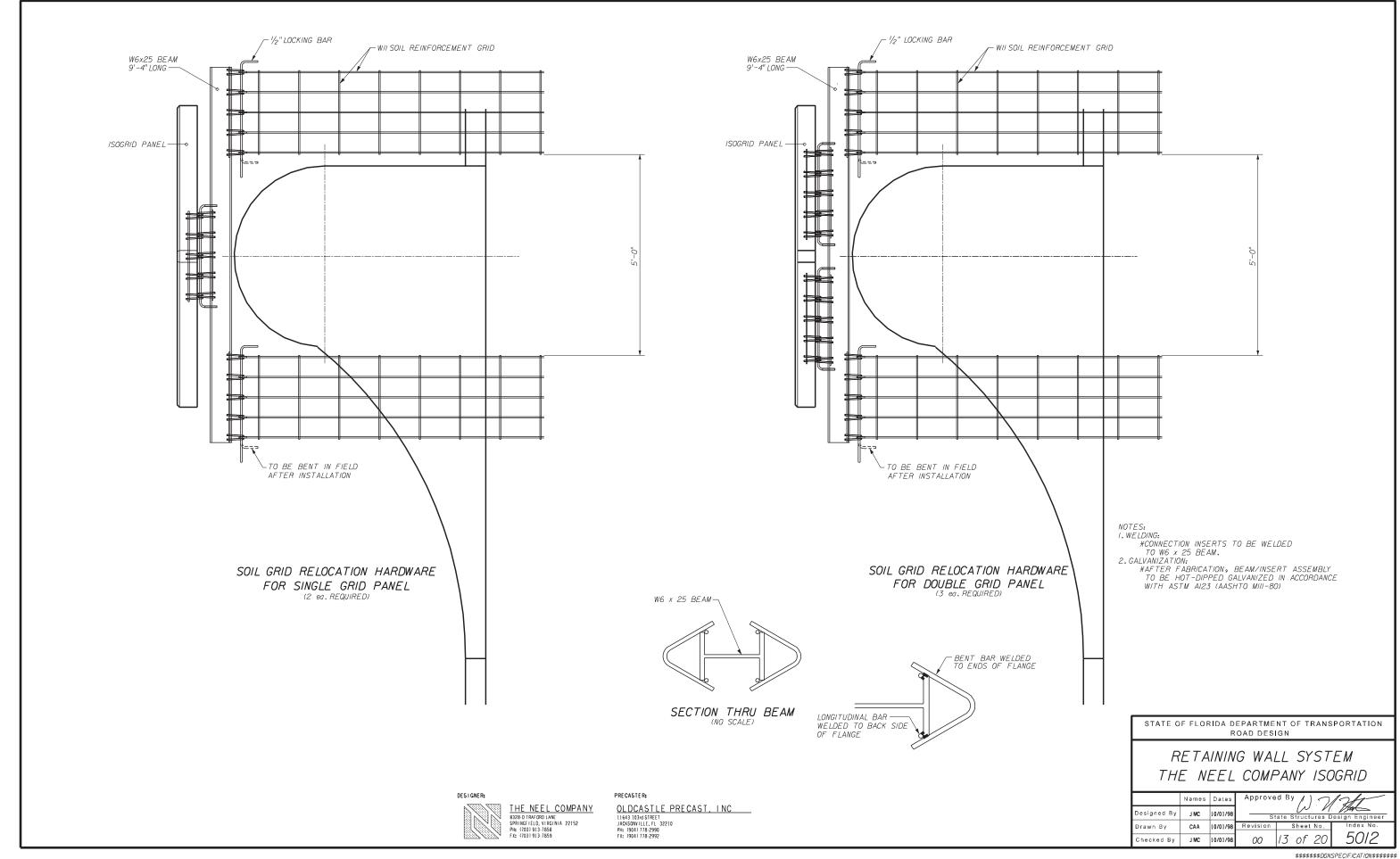
PRECASTER:

OLDCASTLE PRECAST, INC
11643 10344 STREE
JACKSOWILLE, FL 122210
Pik: 19041 778-2990
FX: 19041 778-2992

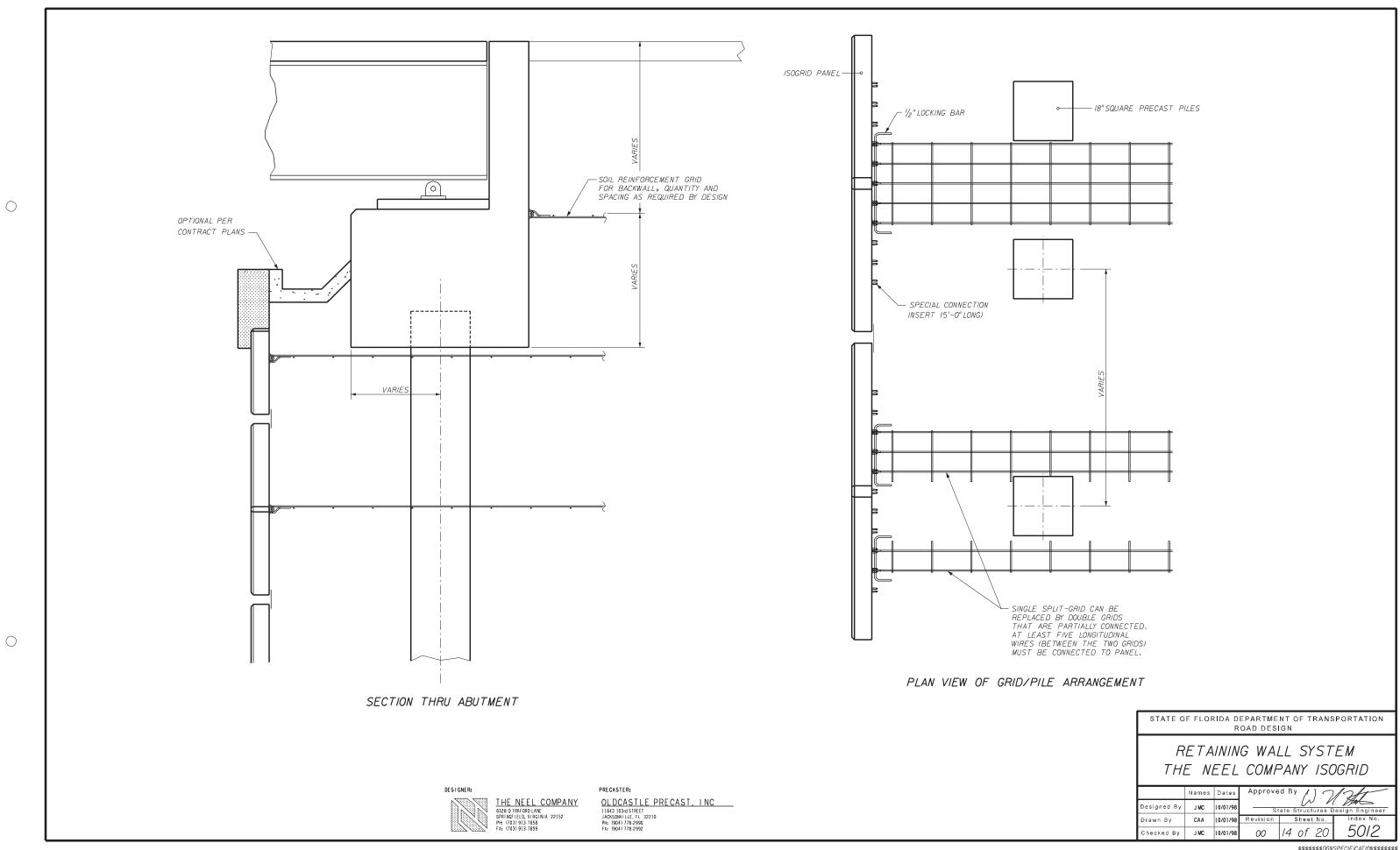
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

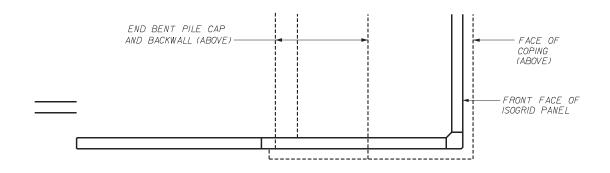
RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

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Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.		
Checked By	J MC	10/01/98	00	12 of 20	<i>5012</i>		

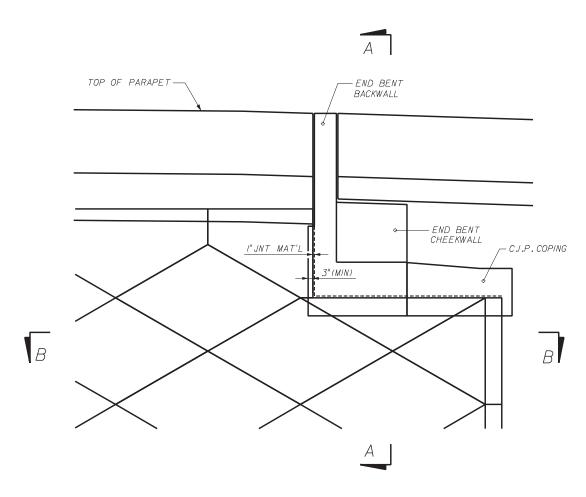


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SECTION B-B STEM / END BENT PILE INTERFACE



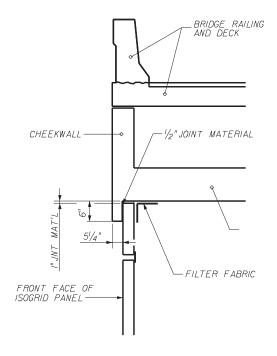
PART ELEVATION SHOWING
WINGWALL / END BENT INTERFACE

DESIGNER:



PRECASTER:

OLDCASTLE PRECAST, INC
11643 103rd STREET
JACKSONVILLE, EL 32210
PR: 1904 178-2990
FX: 17031 913-7859

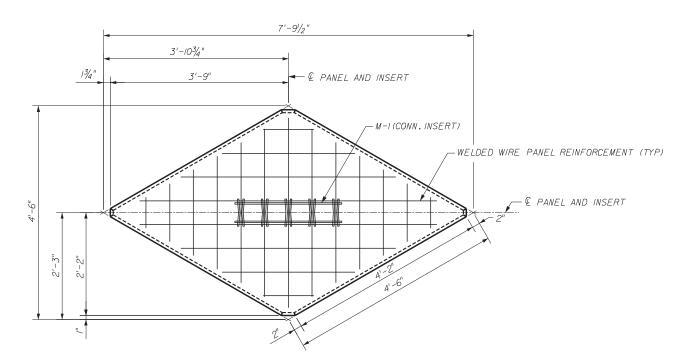


SECTION A-A SECTION THRU PILE CAP

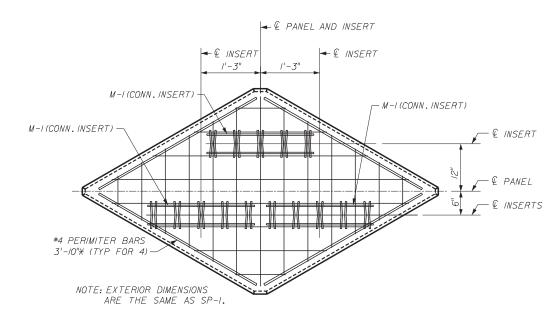
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

	Names	Dates	Approved By					
Designed By	J MC	10/01/98	State Structures Design Engineer					
Drawn By	CAA	10/01/98	Revision	Sheet No.	Index No.			
Checked By	J MC	10/01/98	00	15 of 20	<i>5012</i>			



FULL-SIZE PANEL - ONE GRID ELEVATION (REAR FACE)



SP-3 FULL-SIZE PANEL - THREE GRIDS ELEVATION (REAR FACE)

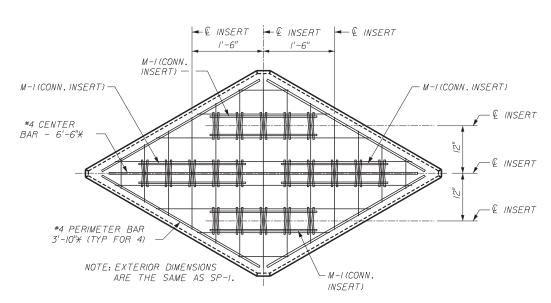


PRECASTER: OLDCASTLE PRECAST, INC

11643 103rd STREET JACKSONVILLE, FL 32210 PH: 19041 778-2990 FX: 19041 778-2992

← & INSERT ← & PANEL ← & INSERT M-1(CONN.INSERT) - M-I(CONN.INSERT) - € PANEL AND INSERTS NOTE: EXTERIOR DIMENSIONS ARE THE SAME AS SP-1.

SP-2 FULL-SIZE PANEL - TWO GRIDS ELEVATION (REAR FACE)



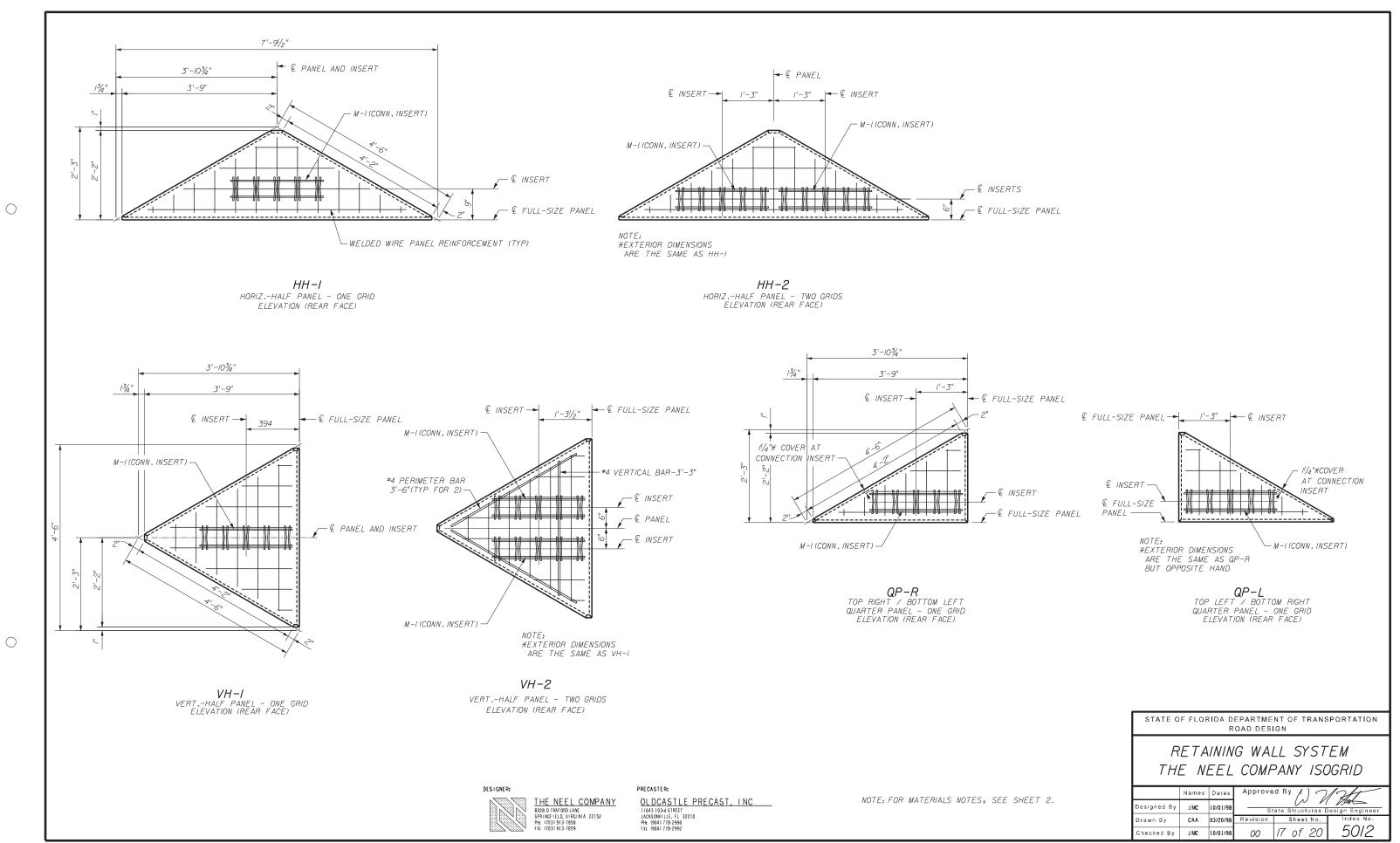
SP-4 FULL-SIZE PANEL - FOUR GRIDS ELEVATION (REAR FACE)

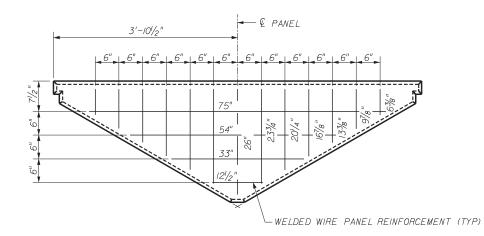
NOTE: FOR MATERIALS NOTES, SEE SHEET 2.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

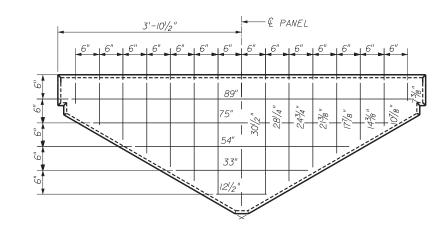
RETAINING WALL SYSTEM THE NEEL COMPANY ISOGRID

	Names	Dates	Approve	d By	12/1		
signed By	J MC	10/01/98	State Structures Design Engineer				
ıwn By	CAA	10/01/98	Revision	Sheet No.	Index No.		
ecked By	J MC	10/01/98	00	16 of 20	<i>5012</i>		



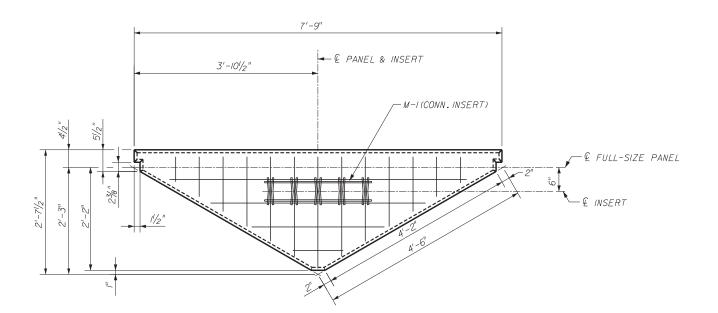


WELDED WIRE MESH PANEL REINFORCEMENT - X-I PANEL ELEVATION (REAR FACE)



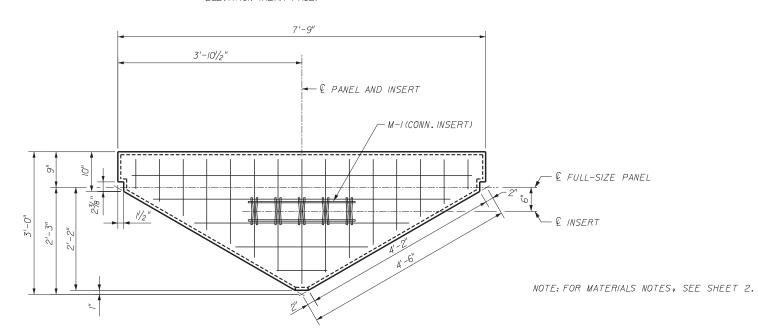
WELDED WIRE MESH PANEL REINFORCEMENT - X-2 PANEL

ELEVATION (REAR FACE)



X−I 4½" RISER - ONE GRID

ELEVATION (REAR FACE)



X-2 9"RISER PANEL - ONE GRID ELEVATION (REAR FACE)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

	Names	Dates	Approved By					
signed By	J MC	10/01/98	State Structures Design Engineer					
awn By	CAA	10/01/98	Revision	Revision Sheet No.				
ecked By	J MC	10/01/98	00	18 of 20	5012			

DESIGNER:

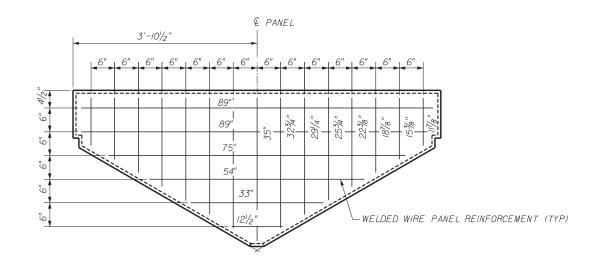


THE NEEL COMPANY
8328-0 TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: (7031913-7858
FX: (7031913-7858

PRECASTER:

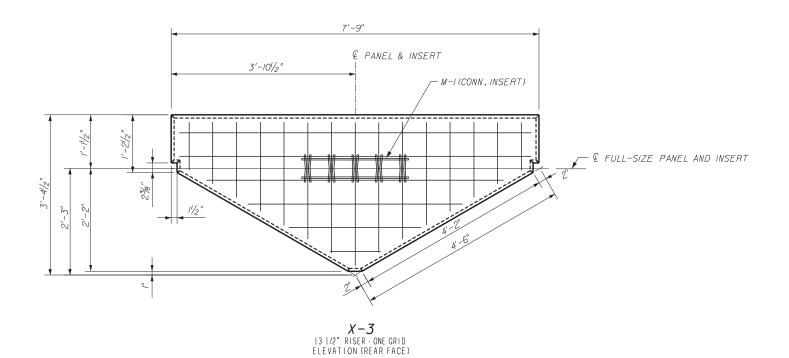
OLDCASTLE PRECAST, INC

11643 103rd STREET JACKSONVILLE, FL 32210 PH: 1904) 778-2990 FX: 1904) 778-2992



WELDED WIRE MESH PANEL REINFORCEMENT - X-3 AND X-3(2) PANELS

ELEVATION (REAR FACE)



T'-9"

W-I(CONN. INSERT)

V INSERT

V-6"

M-I(CONN. INSERT)

V FULL-SIZE PANEL AND INSERTS

NOTE: FOR MATERIALS NOTES, SEE SHEET 2.

X-3 (2) 13 1/2" RISER – TWO GRI ELEVATION (REAR FA)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM
THE NEEL COMPANY ISOGRID

DESIGNER:

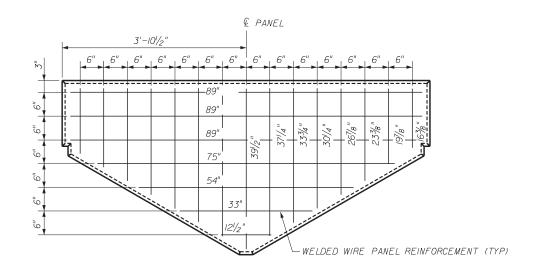


THE NEEL COMPANY
8328-D TRAFORD LANE
SPRINGFIELD, VIRGINIA 22152
PH: 17031 913-7858
FX: 17031 913-7859

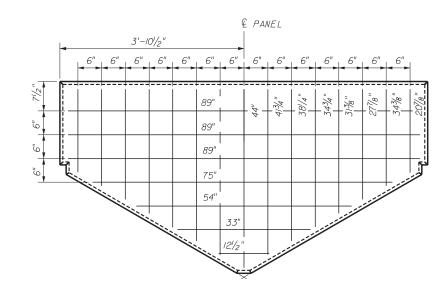
PRECASTER:

OLDCASTLE PRECAST, INC

11643 103rd STREET JACKSONVILLE, FL 32210 PH: 19041 778-2990 FX: 19041 778-2992



WELDED WIRE MESH PANEL REINFORCEMENT - X-4 PANEL ELEVATION (REAR FACE)



WELEDE WIRE MESH PANEL REINFORCEMENT - X-5 PANEL ELEVATION (REAR FACE)

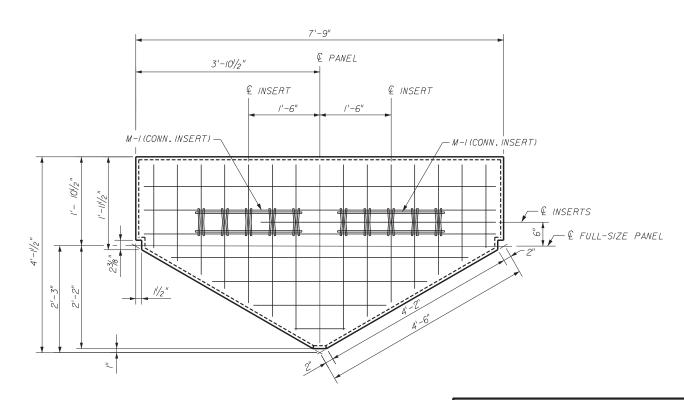
DESIGNER:

THE NEEL COMPANY 8328-D TRAFORD LANE SPRINGFIELD, VIRGINIA 22152 PH: (703) 913-7858 FX: (703) 913-7859

PRECASTER: OLDCASTLE PRECAST, INC
11643 103rd 518EE1
JACKSOWILLE, FL 32210
Pk: 19041 778-2990
FX: 19041 778-2992

@ PANEL 3'-101/2" € INSERT **€** INSERT /'-6" M-1(CONN.INSERT) -— M−I(CONN.İNSERT) G FULL-SIZE PANEL AND INSERT

X-4 18" RISER - TWO GRIDS ELEVATION (REAR FACE)



X-5 22½"RISER - TWO GRIDS ELEVATION (REAR FACE)

NOTE: FOR MATERIALS NOTES, SEE SHEET 2.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM THE NEEL COMPANY ISOGRID

	Names	Dates	Approve	d By /) 7	12/
esigned By	J MC	10/01/98	St	ate Structures D	esign Engineer
rawn By	CAA	10/01/98	Revision	Sheet No.	Index No.
hecked By	J MC	10/01/98	00	20 of 20	5012

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8614 WESTWOOD CENTER DRIVE SUITE 1100, VIENNA VIRGINIA 22182 (703) 821-1175

DESIGN CRITERIA

- DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL WITHIN THE REINFORCED EARTH VOLUME, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED MATERIALS SHALL CONFORM TO THE CONTRACTING AGENCY'S TECHNICAL SPECIFICATIONS (SECTION 548) FOR REINFORCED EARTH WALLS
- 2. SOIL PARAMETERS:

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF FRICTION ANGLE (\$\phi\$), COHESION (c) AND TOTAL UNIT WEIGHT (Y) SHALL BE PROVIDED IN THE SHOP DRAWINGS.

- 3. THE MAXIMUM APPLIED BEARING PRESSURE AT THE FOUNDATION LEVEL IS AS SHOWN ON THE WALL ELEVATIONS FOR EACH DESIGN CASE. IT IS THE RESPONSIBILITY OF THE OWNER TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 4. ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED EARTH VOLUME, AS DETERMINED BY THE ENGINEER, SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL OR OTHERWISE STABILIZED AS DIRECTED BY THE ENGINEER.
- 5. REINFORCING STRIPS FOR REINFORCED EARTH WALLS SHALL BE 131/32" WIDE AND 5/32" THICK, AND SHALL CONFORM TO THE PHYSICAL AND MECHANICAL PROPERTIES OF ASTM A-572 GRADE 65. GALVANIZATION SHALL BE APPLIED IN ACCORDANCE WITH ASTM A-123.
- 6. THE MINIMUM FACTORS OF SAFETY REQUIRED FOR DESIGN

OVERTURNING = 2.0 SLIDING = 1.5INTERNAL PULLOUT = 1.5 (ALLOWABLE DEFORMATION = 0.75 INCH) BEARING CAPACITY = 2.5 OVERALL STABILITY = 1.5 STEEL SOIL REINFORCEMENT = 0.55Fy AT END OF DESIGN LIFE AND 0.50 Fu AT NET SECTION OF BOLTED CONNECTION MAXIMUM PULLOUT FACTOR fx (FOR SAND) = 1.5 (FOR LIMEROCK) = 2.0

WALL CONSTRUCTION

- 7. REINFORCED EARTH WALLS IN CURVES WILL FORM A SERIES OF SHORT CHORDS OF 4'-II" EACH TO MATCH DESIRED WALL ALIGNMENT.
- 8. FOR LOCATION AND ALIGNMENT OF REINFORCED EARTH WALLS, SEE RETAINING WALL CONTROL PLANS.
- 9. IF MANHOLES AND DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN ON WALL ELEVATIONS.
- 10. IF PILES ARE LOCATED WITHIN THE REINFORCED EARTH VOLUME, THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE REINFORCED EARTH WALL UNLESS A METHOD TO PROTECT THE STRUCTURE, WHICH IS ACCEPTABLE TO THE ENGINEER AND THE REINFORCED EARTH COMPANY, AND IS PROPOSED AND APPROVED IN WRITING.

- II. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH SEC 548 TO A LEVEL OF 2" ± ABOVE THE TIE STRIPS EMBEDDED IN THE PANELS. INSTALLATION OF REINFORCING STRIPS SHALL BE PERMITTED ONLY AFTER PLACEMENT AND COMPACTION OF THE BACKFILL MATERIAL HAS REACHED THE REQUIRED LEVEL.
- 12. IF STRUCTURES IN EXCESS OF 20'IN HEIGHT OCCUR, THE FINISHED GRADE IN FRONT OF THE WALL SHALL BE PLACED AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS A HEIGHT OF 20', FINISHED GRADE BACKFILL SHALL BE COMPACTED TO 95% OF AASHTO T-180 UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- 13. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE LOCATION OF ANY GUARDRAIL POSTS BEHIND THE REINFORCED EARTH PANELS PRIOR TO PLACEMENT OF THE TOP LAYER OF REINFORCING STRIPS. INDIVIDUAL STRIPS MAY BE SKEWED UP TO 15°TO AVOID THE POST LOCATIONS IF AUTHORIZED BY THE ENGINEER. ANY DAMAGE DONE TO THE REINFORCING STRIPS DUE TO THE INSTALLATION OF THE GUARDRAIL SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- 14. IF EXISTING OR FUTURE STRUCTURES, PIPES, FOUNDATIONS OR GUARDRAIL POSTS WHICH ARE WITHIN THE REINFORCED EARTH VOLUME INTERFERE WITH THE NORMAL PLACEMENT OF REINFORCING STRIPS AND SPECIFIC DIRECTION HAS NOT BEEN PROVIDED ON THE PLANS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO DETERMINE WHAT COURSE OF ACTION SHOULD BE TAKEN.
- 15. TOP PANELS BENEATH COPING SHALL HAVE #4 DOWELS PROTRUDING FROM THEIR TOP EDGE.
- 16. FOR OTHER INFORMATION PERTAINING TO WALL CONSTRUCTION PLEASE REFER TO THE REINFORCED EARTH CONSTRUCTION MANUAL.
- 17. THE CONTRACTOR IS RESPONSIBLE FOR GRADUALLY DEFLECTING UPPER REINFORCING STRIPS DOWNWARD TO AVOID CONFLICTS WITH PAVING AND SUBGRADE PREPARATION. THE CONTRACTOR'S ATTENTION IS DIRECTED ESPECIALLY TO SITUATIONS WHERE ROADWAY SUPERELEVATION AND/OR SOIL MIXING ARE ANTICIPATED.

MATERIALS NOTES

18. NOMINAL STRIP LENGTHS

THE REINFORCING STRIP LENGTHS SHOWN ON THE PLANS, MEASURED FROM BACK FACE OF PANEL, ARE THE NOMINAL LENGTHS REQUIRED BY CALCULATION. THE ACTUAL FABRICATED STRIP LENGTHS ARE OFTEN LONGER (UP TO 6") DUE TO MANUFACTURING TOLERANCES. THE REQUIRED HORIZONTAL LIMIT OF GRANULAR BACKFILL IS EQUAL TO THE NOMINAL STRIP LENGTH.

19. PANEL FINISH

THE PRECAST PANELS FOR THIS PROJECT SHALL HAVE A PLAIN STEEL FINISH UNLESS OTHERWISE SPECIFIED ON THE RETAINING WALL CONTROL PLANS.

20. NOTE TO CONTRACTORS

ONLY THE FOLLOWING MATERIALS ARE SUPPLIED BY THE REINFORCED EARTH COMPANY:

- PRECAST CONCRETE FACING PANELS
- REINFORCING STRIPS
- BOLT SETS (FOR ATTACHING PANELS TO THE REINFORCING STRIPS)
- BEARING BLOCKS
- RUBBER SHIMS
- FILTER CLOTH AND ADHESIVE (FOR PANEL JOINTS ONLY)

ANY OTHER MATERIALS CALLED FOR IN THE CONTRACT PLANS OR SPECIFICATIONS ARE TO BE SUPPLIED BY THE CONTRACTOR. ANY JOINT MATERIALS SHOWN AT THE INTERFACE OF PRECAST PANELS AND CAST-IN-PLACE CONCRETE STRUCTURES ARE TO BE SUPPLIED BY THE ERECTION CONTRACTOR. ALL SANDBLASTING, PAINTING, SEALERS OR OTHER SPECIAL APPLIED COATINGS ARE ALSO SUPPLIED/INSTALLED BY THE CONTRACTOR IN THE FIELD FOLLOWING PANEL ERECTION.

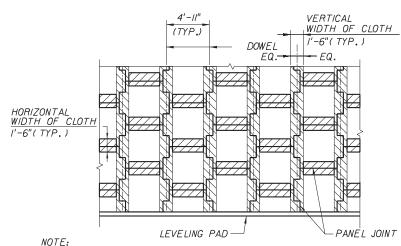
- 21. THE REINFORCED EARTH COMPANY SUPPLIES PRECAST CONCRETE FACING PANELS AND ACCESSORIES TO BE USED IN CONJUNCTION WITH OTHER MATERIALS IN THE CONSTRUCTION OF THE REINFORCED EARTH ® RETAINING WALLS DETAILED HEREIN. THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL FURNISHED BY THE REINFORCED EARTH COMPANY IS INTENDED TO PROVIDE A GENERAL EXPLANATION OF THE SYSTEM. IT IS THE CONTRACTOR'S OBLIGATION TO DEVISE AND EXECUTE A PROJECT SPECIFIC ERECTION SEQUENCE, PANEL UNLOADING, HANDLING AND BRACING SYSTEM, AND FALL PROTECTION SYSTEM. THE BRACING SYSTEM SHOWN IN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL IS GENERAL IN NATURE AND DOES NOT ACCOUNT FOR PROJECT SPECIFIC CRITERIA. COMPLIANCE WITH THE GUIDELINES IN THIS MANUAL DOES NOT RELIEVE THE CONTRACTOR OF ITS RESPONSIBILITY TO ADHERE TO THE PROJECT PLANS, SPECIFICATIONS AND CONTRACT DOCUMENTS OR COMPLIANCE WITH ALL FALL PROTECTION, SAFETY, LAWS, STANDARDS AND PROCEDURES AT THE JOBSITE. CONTRACTORS SHOULD TAKE SPECIAL PRECAUTIONS TO PREVENT THE PANELS FROM SHIFTING OR FALLING DURING THE ERECTION PROCESS.
- 22. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY THE OWNER. ON THE BASIS OF THIS INFORMATION, THE REINFORCED EARTH COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.
- 23. THESE DRAWINGS ARE CERTIFIED WITH RESPECT TO THE INTERNAL STABILITY OF REINFORCED EARTH STRUCTURES ONLY
- 24. THIS DRAWING CONTAINS INFORMATION PROPRIETARY TO THE REINFORCED EARTH COMPANY, AND IS BEING FURNISHED FOR THE USE IN CONNECTION WITH FDOT PROJECTS ONLY, AND THE INFORMATION CONTAINED HEREIN IS NOT TO BE TRANSMITTED TO ANY OTHER ORGANIZATION UNLESS SPECIFICALLY AUTHORIZED IN WRITTING BY THE REINFORCED EARTH COMPANY. THE REINFORCED EARTH COMPANY IS EXCLUSIVE LICENSEE IN THE UNITED STATES UNDER PATENTS ISSUED TO HENRY VIDAL, AND THE FURNISHING OF THIS DRAWING DOES NOT CONSTITUTE AN EXPRESSED OR IMPLIED LICENSE UNDER THE VIDAL PATENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM REINFORCED EARTH COMPANY REINFORCED EARTH WALL

Approved By Names Dates Designed By rawn By 1 of 14 hecked By 00

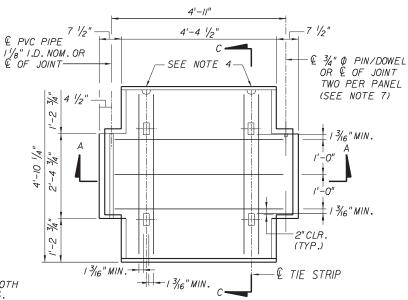
THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY CRUCIFORM AND SQUARE PANELS



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NOTE:
STRIPS OF FILTER CLOTH SHALL BE PLACED ON BACK FACE OF PANEL,
OVER PANEL JOINTS. FILTER CLOTH SHALL BE ADHERED TO BACK FACE
OF PANELS USING AN ADHESIVE COMPOUND SUPPLIED BY THE REINFORCED
EARTH COMPANY. ADHESIVE SHALL BE APPLIED TO PANEL THEN FILTER CLOTH
(CARTHAGE MILLS TYPE FX-40HS OR EQUAL) SHALL BE APPLIED TO PANELS.

FILTER CLOTH DETAIL PARTIAL ELEVATION - BACK FACE

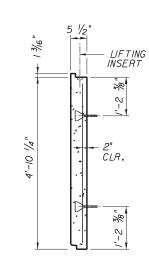


PANEL TYPE "A" WITH R4 REINFORCEMENT FRONT VIEW

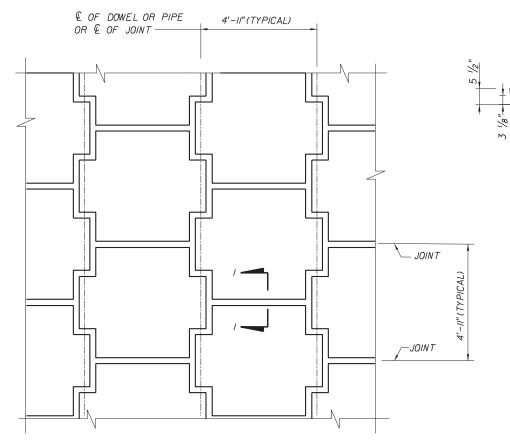
SECTION A-A

#3 BAR; 4'-6" LONG (TYP.)

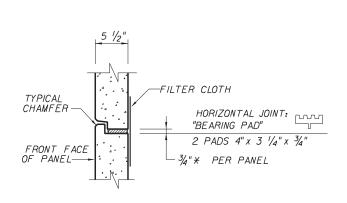
#4 BAR; 4'-9" LONG (TYP) HORIZONTAL (TYP.)



SECTION C-C



TYPICAL PANEL LAYOUT
PARTIAL ELEVATION - FRONT FACE



SECTION I-I

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

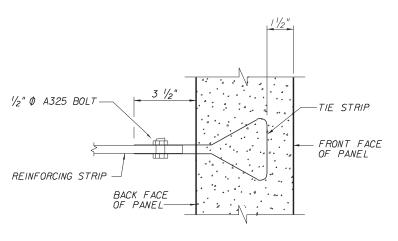
CRUCIFORM PANELS

PANEL THICKNESS	REINFORCEMENT DESIGNATION	* PANEL REINFORCEMENT (IN ²)	MAXIMUM ALLOWABLE HORIZONTAL STRESS AT FACING (KPA)
5 ½"	R4	0.44 VERTICAL 0.58 HORIZONTAL	1.19
	R6	0.66 VERTICAL 0.78 HORIZONTAL	1.46
	R7	I.18 VERTICAL I.78 HORIZONTAL	2.58

* TOTAL AREA OF STEEL REQUIRED PER "TYPE A" PANEL.

NOTES:

- I. REINFORCING STEEL TO BE A615 GRADE 60.
- 2. 3/8" x 3/8" CHAMFER SHALL BE PROVIDED ON ALL EXPOSED EDGES (FRONT FACE ONLY).
- 3. ALL PANEL TYPES AND OTHER RELATED ELEMENTS WILL BE DETAILED ON SHOP DRAWINGS.
- 4. ALL PANELS SHALL HAVE TWO LIFTING INSERTS OF ONE TON CAPACITY EACH.
- 5. PANEL DESIGN THICKNESS IS 5 1/2". THICKNESS
 OF CONCRETE MUST INCREASE TO ACCOMMODATE ANY
 ARCHITECTURAL SURFACE FINISH THAT MAY BE SPECIFIED.
- 6. ACTUAL PANEL REINFORCEMENT FOR ALL PANEL TYPES ON THIS PROJECT IS DESIGNATED ABOVE. R4 ILLUSTRATED FOR INFORMATION ONLY.
- 7. EACH 3/4" © DOWEL SHALL HAVE A TYP.LENGTH OF 10". DOWELS MAY BE GALVANIZED STEEL OR PVC ROD. A SINGLE FULL LENGTH DOWEL MAY BE USED AT THE DISCRETION OF THE MANUFACTURER.



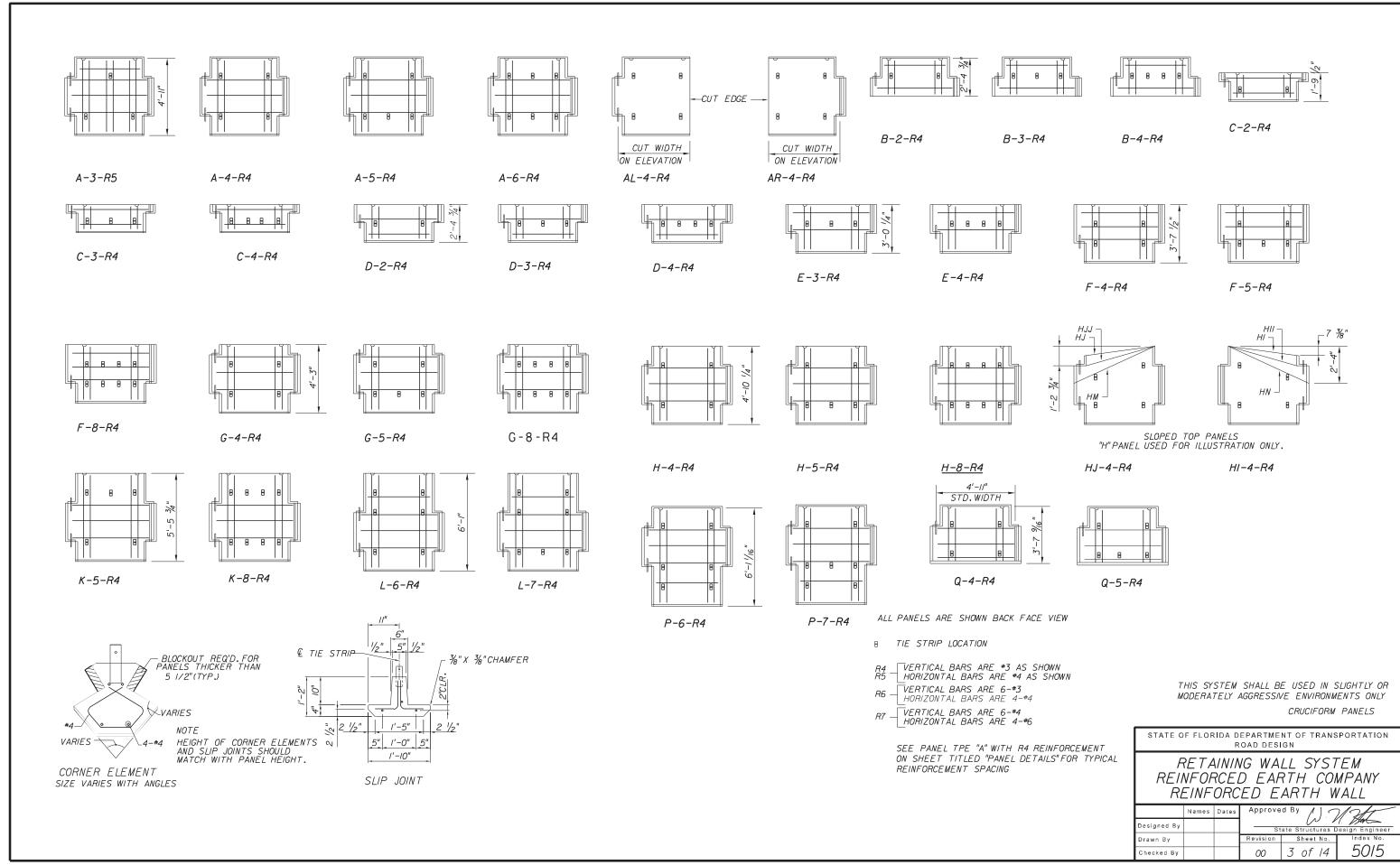
CONNECTION DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

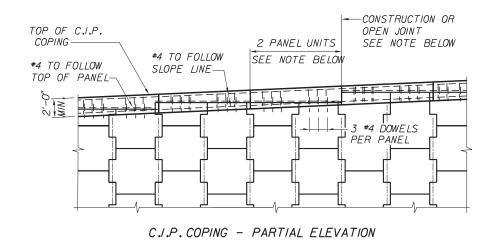
RETAINING WALL SYSTEM
REINFORCED EARTH COMPANY
REINFORCED EARTH WALL

	Names	Dates	Approve	d By	12/1
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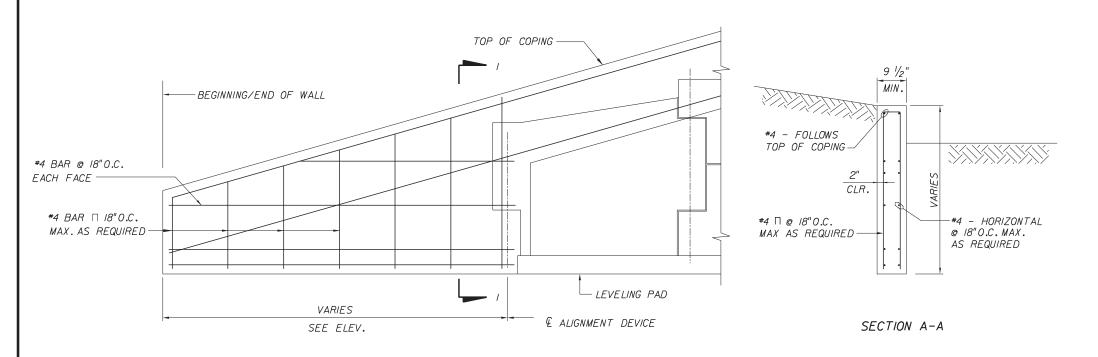


TOP OF LEVEL-UP TOP OF PRECAST CONCRETE--1/2"OPEN JOINT COPING. 10'-1/2" to the total of th H 47/1/1/VY

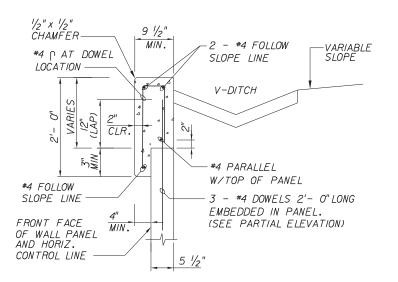
PRECAST COPING PARTIAL ELEVATION

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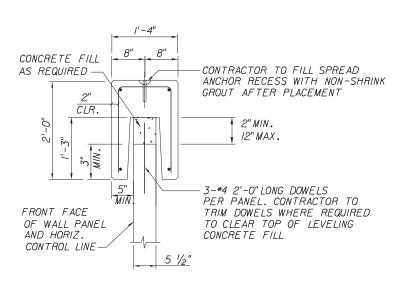
I/2-INCH OPEN JOINTS IN COPING SHALL BE AT 6 PANEL INTERVALS AND COINCIDE APPROXIMATELY WITH © OF ALIGNMENT PINS. REINFORCING STEEL SHALL BE STOPPED 2"SHORT OF EITHER SIDE OF THE JOINTS. CONSTRUCTION JOINTS IN BETWEEN THE OPEN JOINTS SHALL BE PROVIDED AT 2 PANELS INTERVALS.



COPING ENCLOSURE DETAIL



C.I.P. CONC. COPING W/DITCH



PRECAST COPING SECTION

NOTE: STANDARD COPING UNIT IS 10' LONG WITH SQUARE ENDS.

> THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

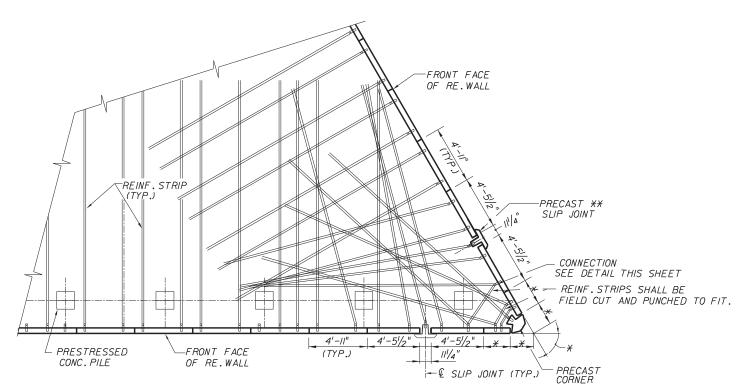
> > CRUCIFORM AND SQUARE PANELS

ROAD DESIGN RETAINING WALL SYSTEM REINFORCED EARTH COMPANY REINFORCED EARTH WALL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

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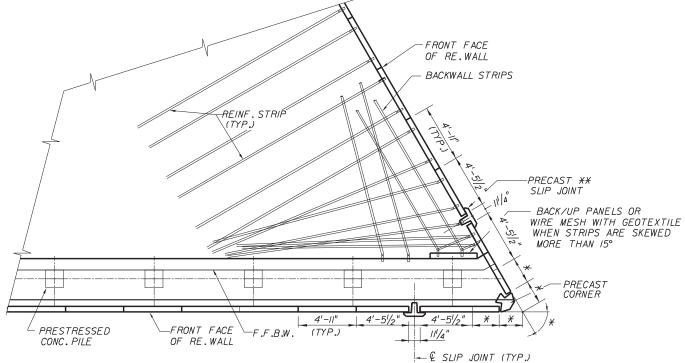
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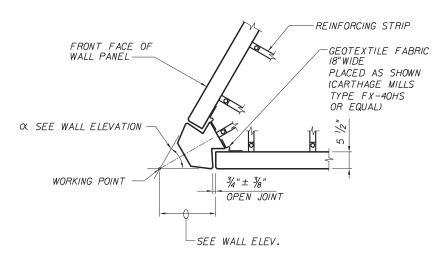
EXAMPLE ACUTE CORNER - SKEWED STRIPS UNDER PILE CAP

NOTE:

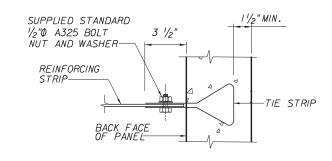
* - DIMENSION OR ANGLE VARIES, SEE WALL ELEVATION ** - SLIP JOINTS ARE NOT REQUIRED FOR SQUARE PANELS



EXAMPLE ACUTE CORNER - SKEWED STRIPS AT ABUTMENT LEVEL



ACUTE CORNER ELEMENT DETAIL



CONNECTION DETAIL

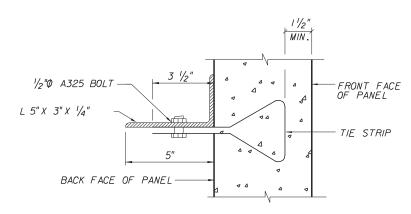
THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

CRUCIFORM AND SQUARE PANELS

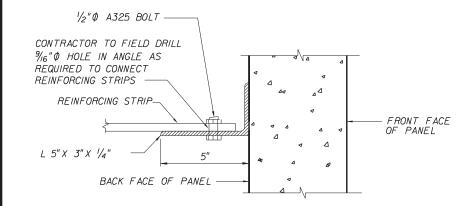
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
REINFORCED EARTH COMPANY
REINFORCED EARTH WALL

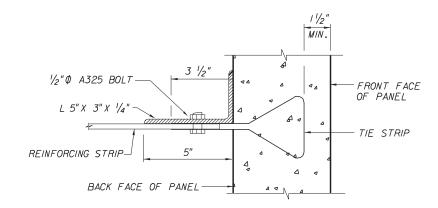
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CONNECTION DETAIL
ANGLE BOLTED TO TIE STRIP

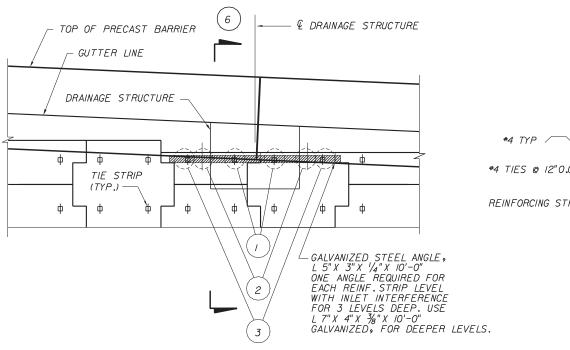


2 CONNECTION DETAIL
SHIFTED REINF.STRIP BOLTED TO ANGLE



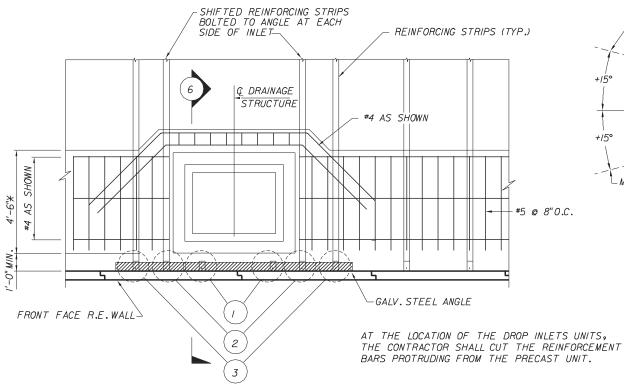
CONNECTION DETAIL

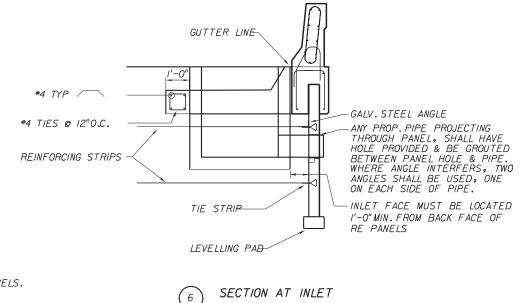
ANGLE BOLTED TO TIE STRIP WITH REINF. STRIP

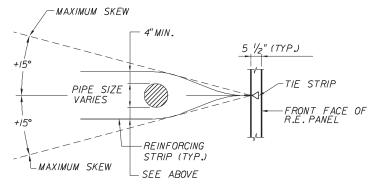


PARTIAL ELEVATION

PARTIAL PLAN







NOTE : BEND TO BE AS GRADUAL AS POSSIBLE.

7 TYPICAL STRIP BENDING DETAIL AT ANY PROPOSED & EXISTING PIPES

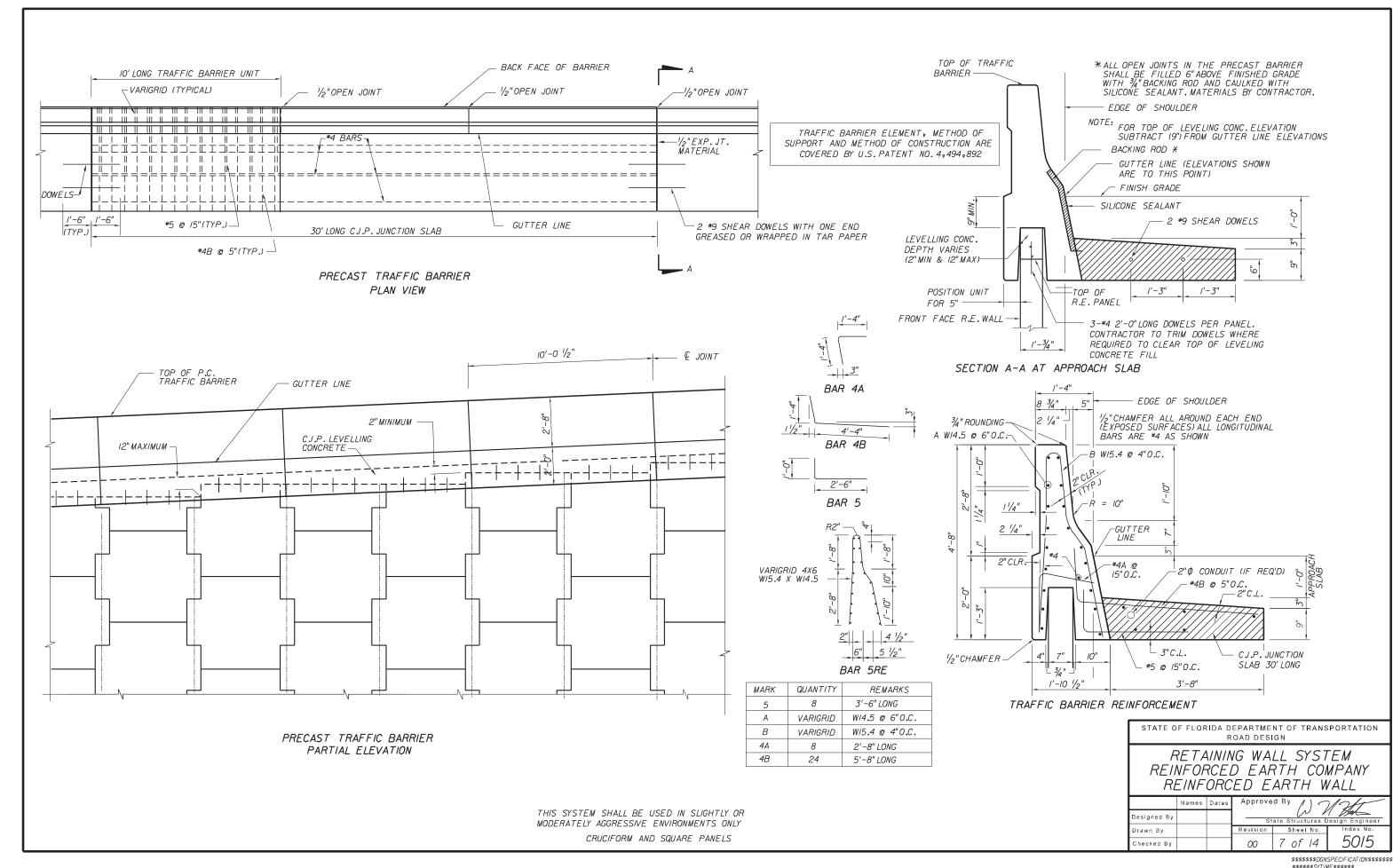
THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY CRUCIFORM AND SQUARE PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

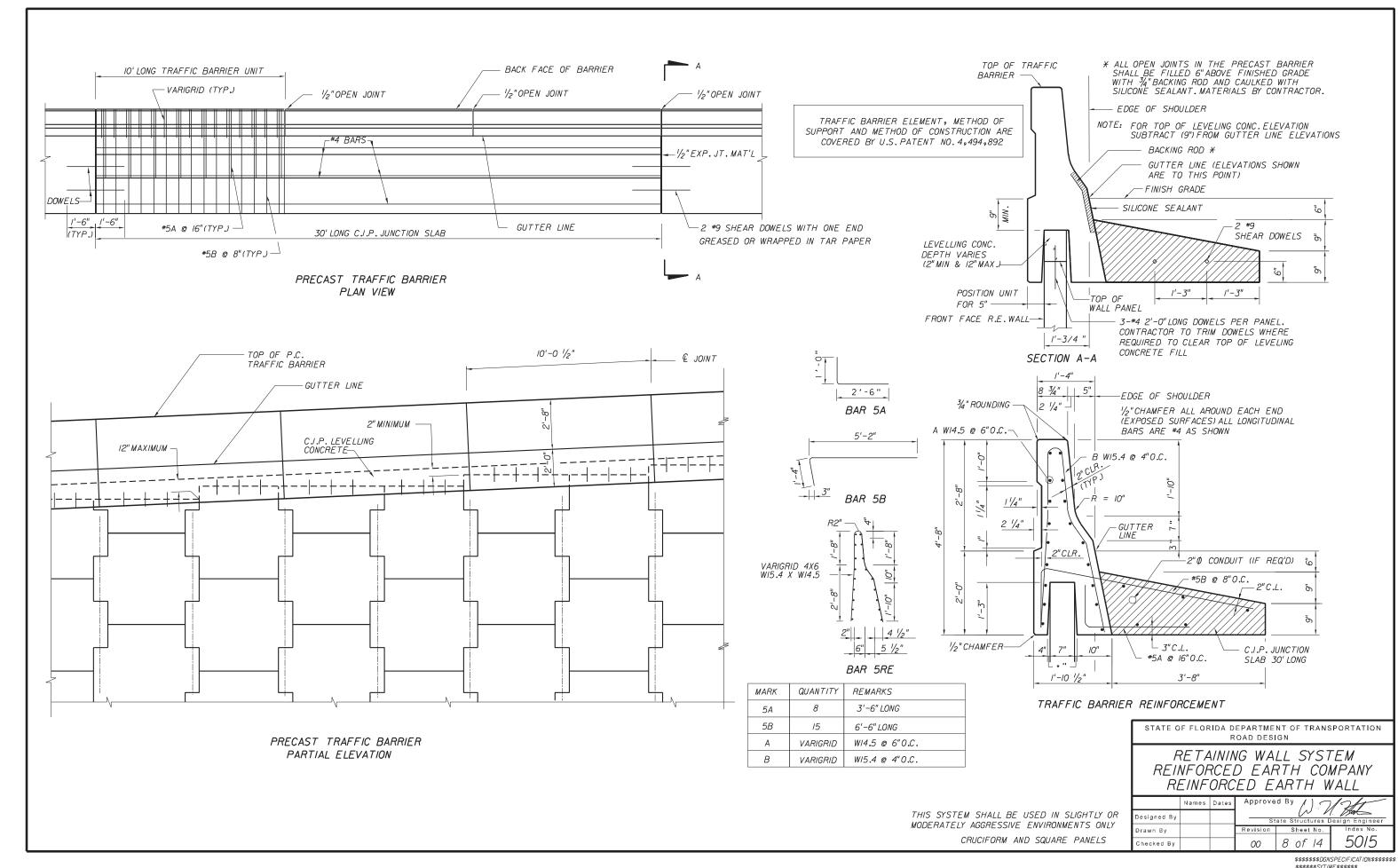
RETAINING WALL SYSTEM
REINFORCED EARTH COMPANY
REINFORCED EARTH WALL

	Names	Dates	Approve		12/	
Designed By			State Structures Design Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	6 of 14	5015	

\$\$\$\$\$\$\$DGNSPECIFICATION\$TIME\$\$\$\$\$\$

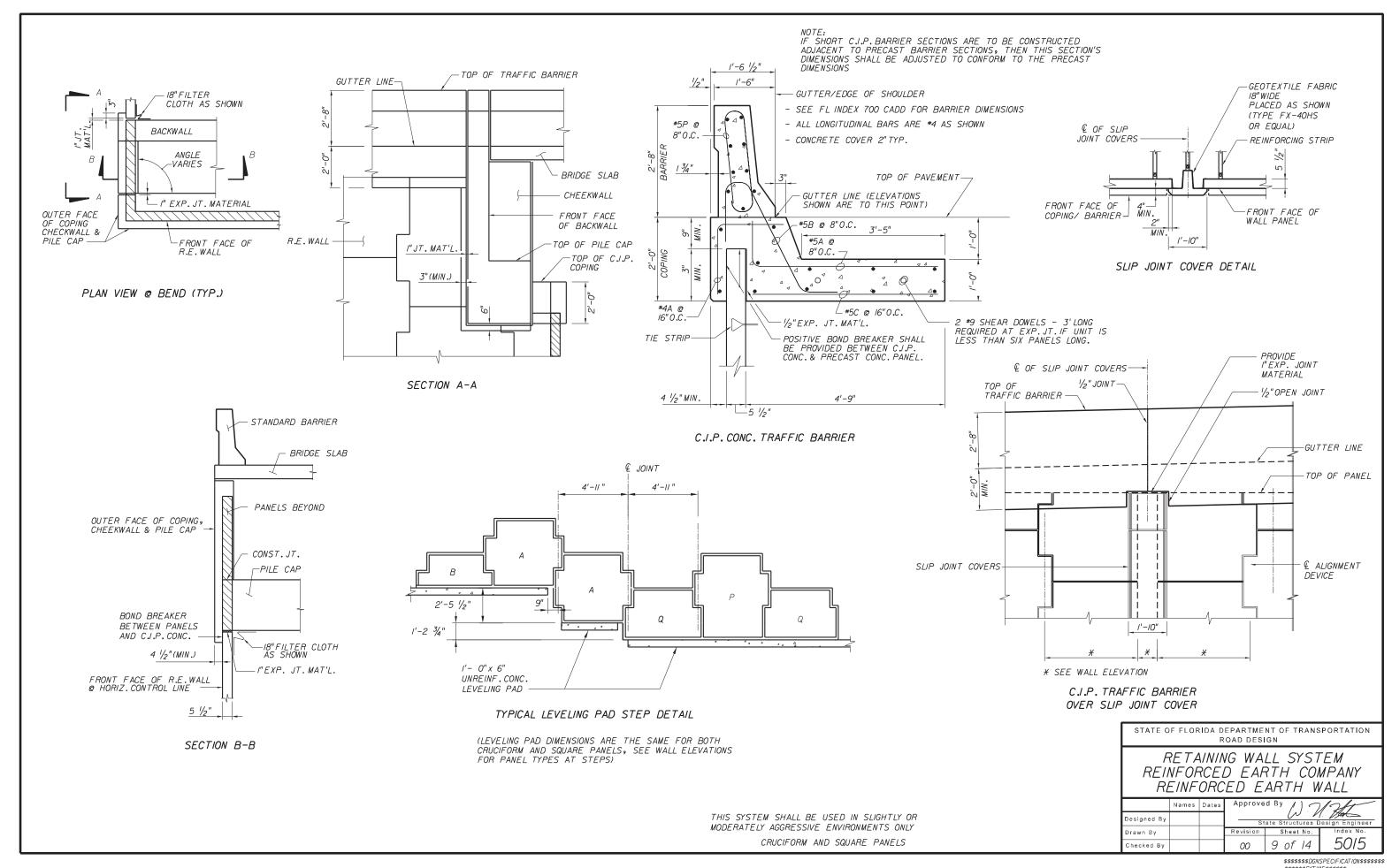


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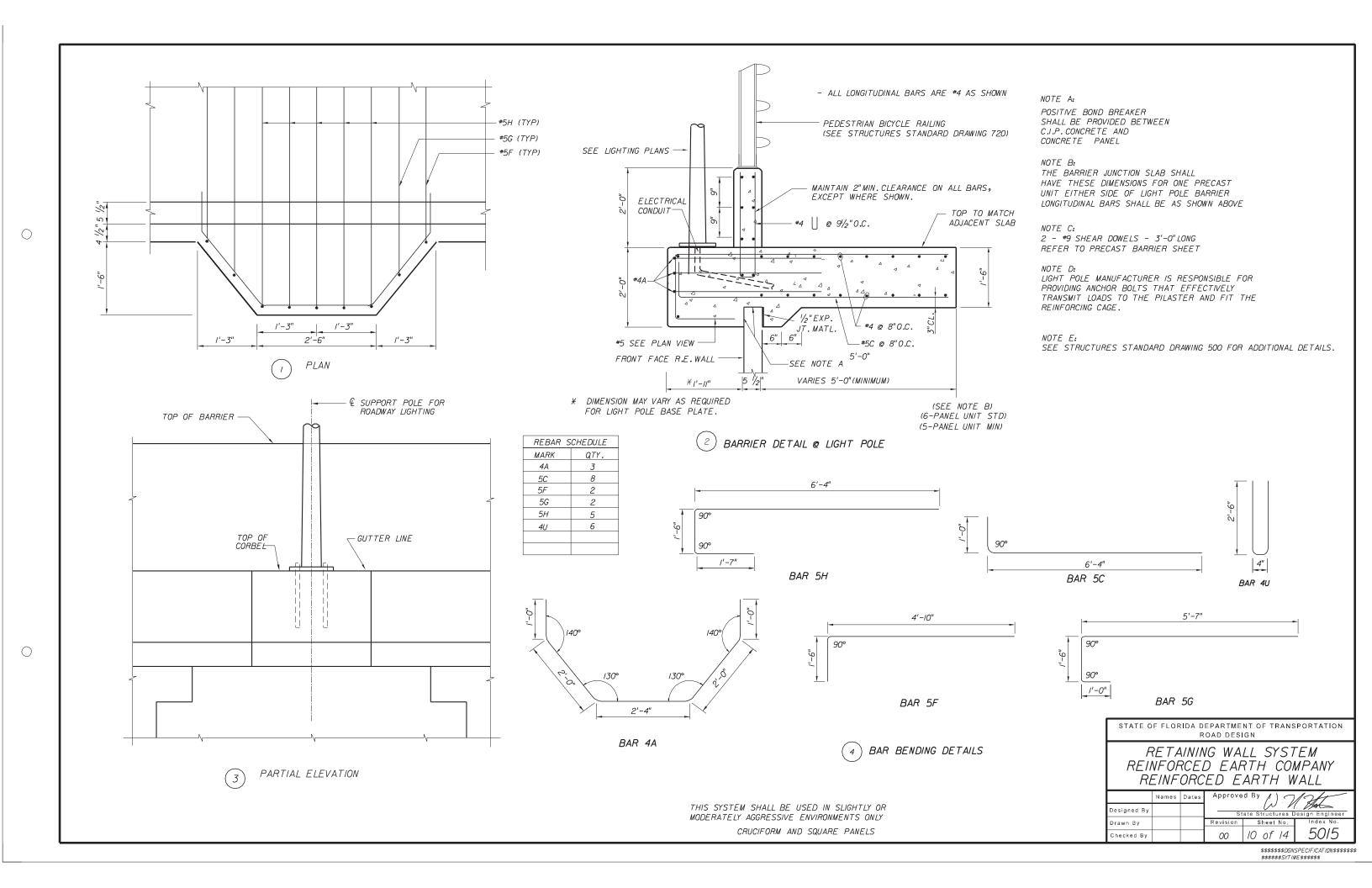
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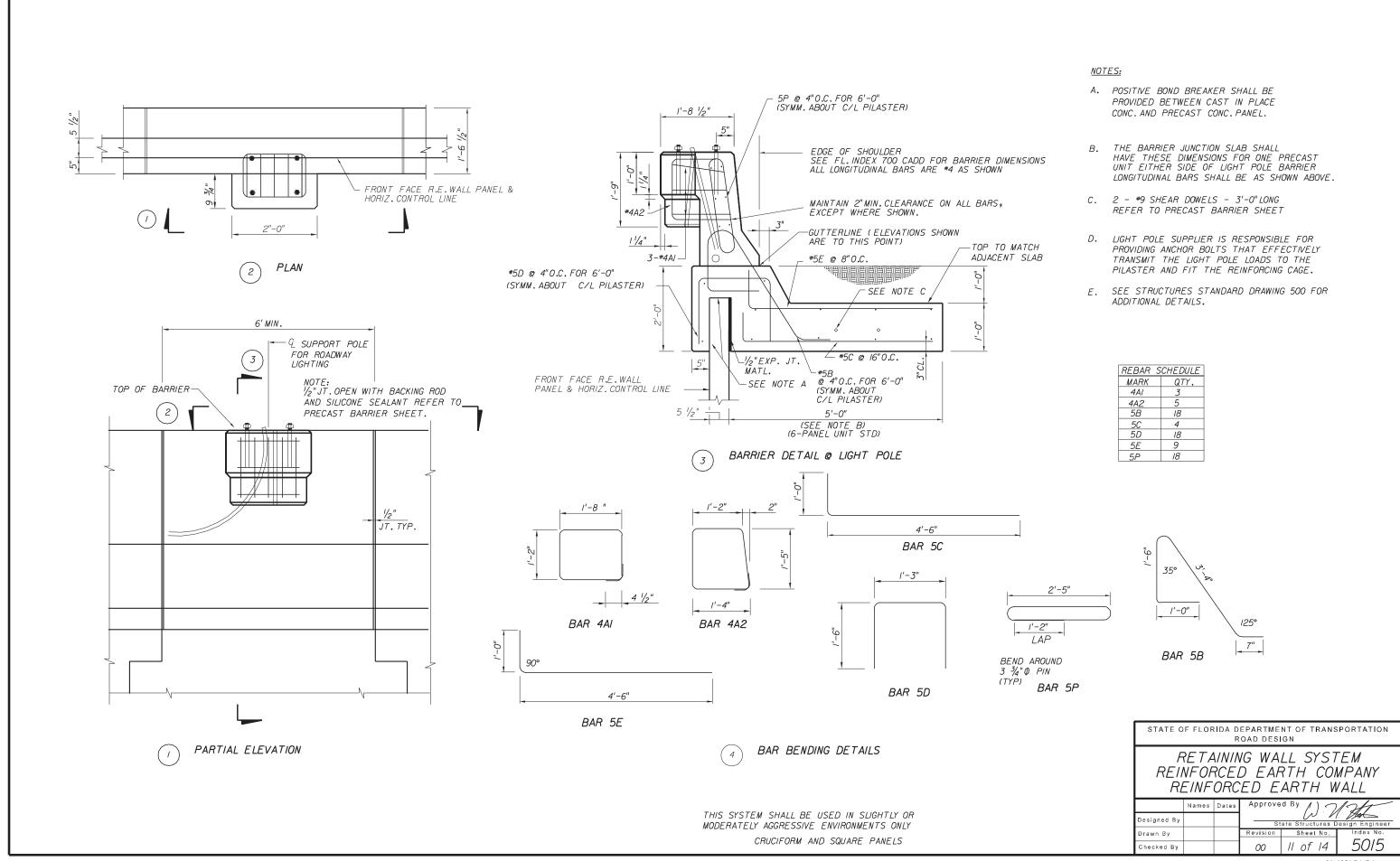
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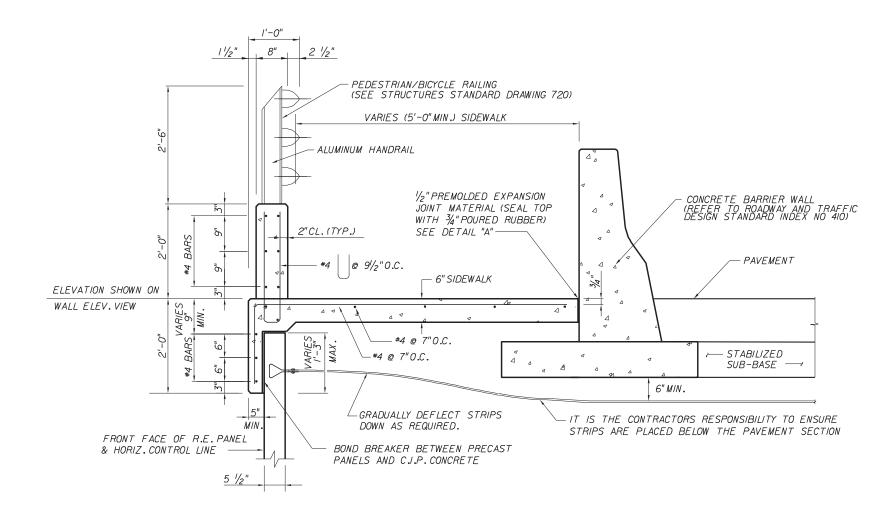


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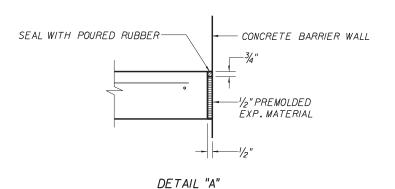
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C.I.P. PARAPET DETAIL W/ HANDRAIL



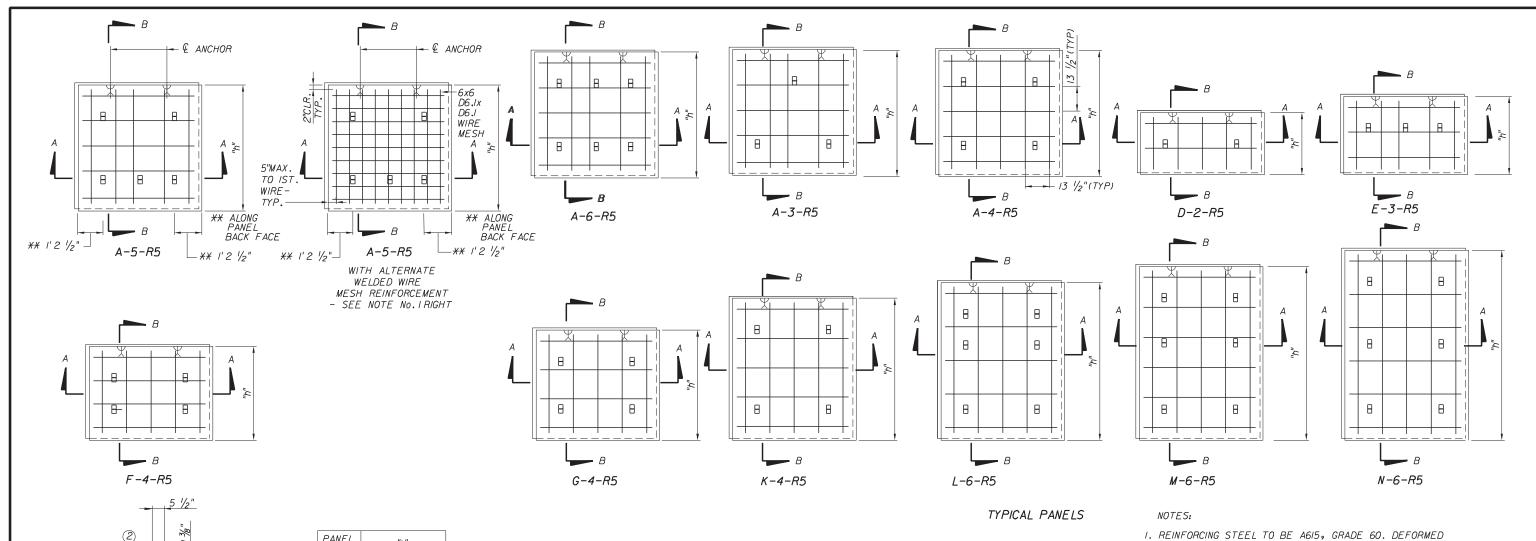
THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

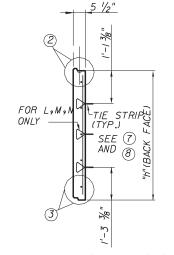
CRUCIFORM AND SQUARE PANELS

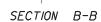
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

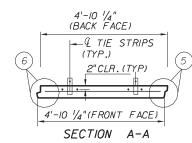
RETAINING WALL SYSTEM REINFORCED EARTH COMPANY REINFORCED EARTH WALL

	Names	Dates	Approve	d By	12/1
Designed By			St	tate Structures E	Oesign Engineer
Drawn By			Revision	Sheet No.	Index No.
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PANEL TYPE	"h"
Α	4'-10 1/4"
D	2'-4 3/4"
E	3'-0 1/4"
F	3'-7 1/2"
G	4'-3"
К	5'-5 ¾"
L	6'-1"
М	6'-8 1/2"
N	7'-4"

NOTE: CONCRETE COVER ON ALL REINFORCEMENT TO BE 2" M/N.

PANEL THICKNESS	REINFORCEMENT DESIGNATION	PANEL REINFORCEMENT A ₅	MAXIMUM ALLOWABLE HORIZONTAL STRESS AT FACING (KSF)	
	R5	5-#3 VERTICAL 5-#3 HORIZONTAL	1.19	
5 1/2"	7.5	ALTERNATE 6 x 6 D6.1 x D6.1		
(MIN.)	07	7-#3 VERTICAL 6-#3 HORIZONTAL		
	R7	ALTERNATE 6 x 6 D8.5 x D8.5	1.78	

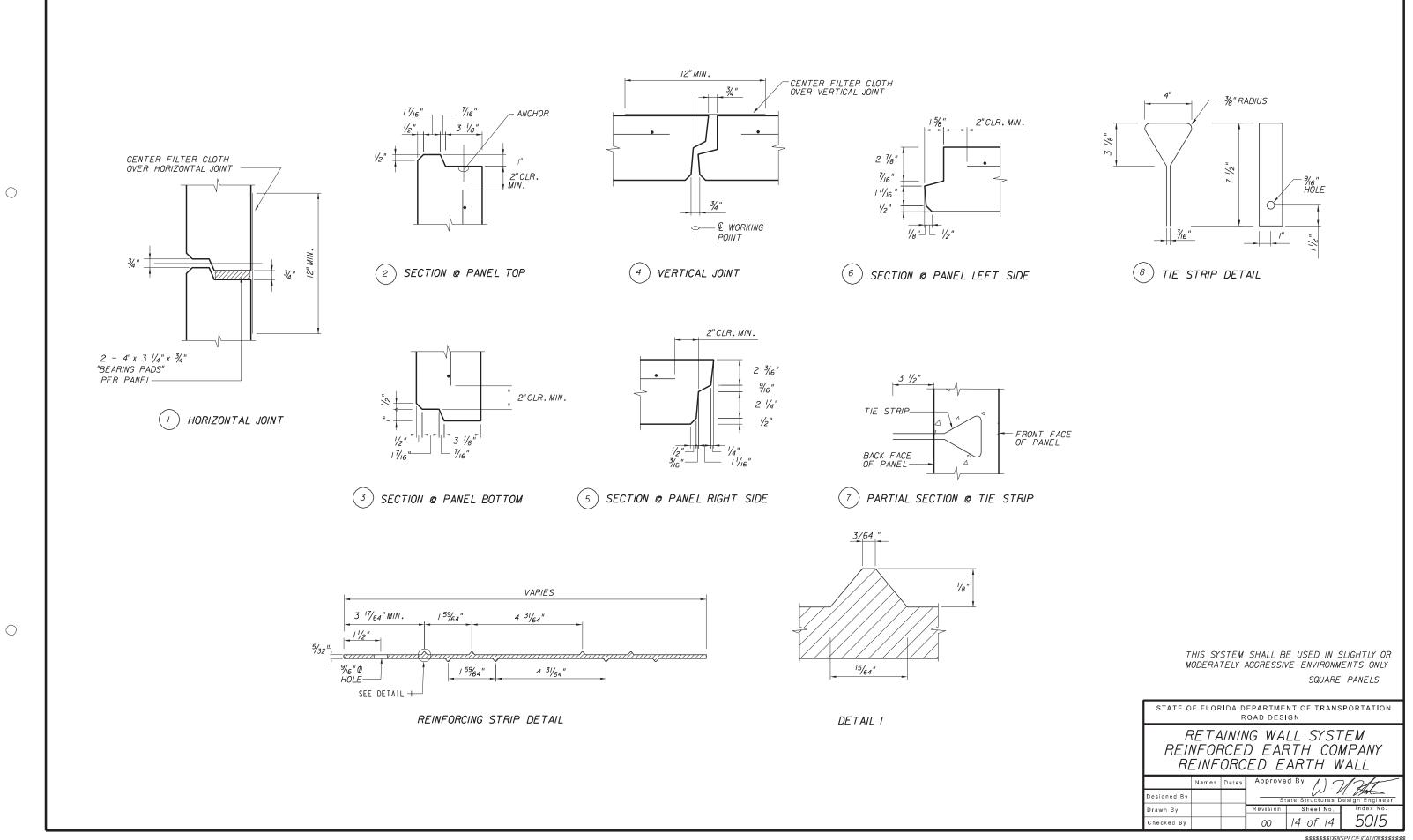
- WELDED WIRE MESH (ASTM A497) MAY BE SUBSTITUTED FOR REBARS. DEFORMED WELDED MESH REQUIREMENTS FOR PANEL "A" IS SHOWN IN THIS SHEET, MESH FOR OTHER PANEL TYPES SHALL BE DETERMINED BASED ON PANEL SHAPE MESH STYLE, AND MINIMUM EDGE CLEAR DISTANCES SHOWN ON THIS SHEET.
- 2. 1/2" x 1/2" CHAMFER SHALL BE PROVIDED ON ALL EXPOSED EDGES (FRONT FACE ONLY).
- 3. ALL PANEL TYPES AND OTHER RELATED ELEMENTS WILL BE DETAILED ON PANEL SHOP DRAWINGS.
- 4. ALL PANELS SHALL HAVE TWO I TON ANCHORS.
- 5. PANEL DESIGN THICKNESS IS 5 1/2" THICKNESS OF CONCRETE MUST INCREASE TO ACCOMMODATE ANY ARCHITECTURAL SURFACE FINISH THAT MAY BE SPECIFIED.
- 6. ACTUAL LOCATION OF REBARS WILL BE ADJUSTED TO ACCOMMODATE PANEL CASTING. MINIMUM 1 3/6" CLEARENCE IS REQUIRED BETWEEN REBARS & TIE-STRIPS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN RETAINING WALL SYSTEM

REINFORCED EARTH COMPANY REINFORCED EARTH WALL

	Names	Dates	Approve	d By /) 7	12/	
Designed By			State Structures Design Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	13 of 14	l <i>5015</i>	

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8614 WESTWOOD CENTER DRIVE SUITE 1100, VIENNA, VIRGINIA 22182 (703) 821-1175

DESIGN CRITERIA

- 1. DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL BEHIND THE PRECAST TECHWALL, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED MATERIALS SHALL CONFORM TO THE SPECIFICATIONS FOR TECHWALL.
- 2. SOIL PARAMETERS:

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF \$\Phi\$, \$C\$ AND \$\footnote{S}\$ SHALL BE PROVIDED IN THE SHOP DRAWINGS.

- 3. THE MAXIMUM APPLIED BEARING PRESSURE AT THE TOE OF THE TECHWALL IS AS SHOWN ON THE WALL ELEVATIONS FOR EACH DESIGN CASE.

 IT IS THE RESPONSIBILITY OF THE OWNER TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 4. ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE CAST-IN-PLACE FOOTING, AS DETERMINED BY THE ENGINEER, SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL OR OTHERWISE STABILIZED AS DIRECTED BY THE ENGINEER.
- 5. THE MINIMUM FACTORS OF SAFETY REQUIRED FOR DESIGN

OVERTURNING = 2.0

SLIDING = 1.5

BEARING CAPACITY = 2.5

OVERALL STABILITY = 1.5

REINFORCING STEEL DESIGN SHALL BE IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES AND FDOT DESIGN GUIDELINES NO. 625-020-150B.

WALL CONSTRUCTION

- 6. FOR LOCATION AND ALIGMENT OF TECHWALL, SEE RETAINING WALL CONTROL PLANS
- 7. TECHWALLS IN CURVES WILL FORM A SERIES OF SHORT CHORDS OF 8.00' EACH TO MATCH DESIRED WALL ALIGNMENT.
- 8. IF MANHOLES AND DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN ON WALL ELEVATIONS.
- 9. IF PILES ARE LOCATED WITHIN THE TECHWALL RETAINED VOLUME, THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE TECHWALL UNLESS A METHOD IS USED TO PROTECT THE STRUCTURE, WHICH IS ACCEPTABLE TO THE ENGINEER AND THE REINFORCED EARTH COMPANY, AND IS PROPOSED AND APPROVED IN WRITING.

- IO. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH SECTION 548 OF THE FLORIDA DOT SPECIFICATIONS.
- II. IF STRUCTURES IN EXCESS OF 20' IN HEIGHT OCCUR, THE FINISHED GRADE IN FRONT OF THE WALL SHALL BE PLACED AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS A HEIGHT OF 20'. FINISHED GRADE BACKFILL SHALL BE COMPACTED TO 95% OF ASSHTO T-180, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- 12. TECHWALL PANELS TO BE FINISHED WITH COPING SHALL HAVE **4 DOWELS PROTRUDING FROM THEIR TOP EDGE.
- 13. FOR OTHER INFORMATION PERTAINING TO WALL CONSTRUCTION PLEASE REFER TO THE REINFORCED EARTH CONSTRUCTION MANUAL FOR TECHWALL.
- 14. IF UNDERDRAIN IS SHOWN, THE FLOWLINE AND OUTLETS SHALL BE AS PER THE CONTRACT PLANS.

MATERIALS NOTES

15. PANEL FINISH

THE PRECAST PANELS FOR THIS PROJECT SHALL HAVE A PLAIN STEEL FORM FINISH UNLESS OTHERWISE SPECIFIED IN THE CONTROL PLANS.

- 16. ONLY THE FOLLOWING MATERIALS ARE SUPPLIED BY THE REINFORCED EARTH COMPANY:
 - PRECAST CONCRETE FACING PANELS
 - GEOCOMPOSITE TERRADRAIN IOIOR EQUIVALENT (FOR PANEL JOINTS ONLY)
 - LIFTING HARDWARE FOR HANDLING PRECAST PANELS. (ON LOAN BASIS)
 - PANEL LEVELLING BOLTS AND PLATES.

ANY OTHER MATERIALS CALLED FOR IN THE CONTRACT PLANS OR SPECIFICATIONS ARE TO BE SUPPLIED BY THE CONTRACTOR. ANY JOINT MATERIALS SHOWN AT THE INTERFACE OF PRECAST PANELS AND CAST-IN-PLACE CONCRETE STRUCTURES ARE TO BE SUPPLIED BY THE ERECTION CONTRACTOR. ALL SANDBLASTING, PAINTING, SEALERS OR OTHER SPECIAL APPLIED COATINGS ARE ALSO SUPPLIED/INSTALLED BY THE CONTRACTOR IN THE FIELD FOLLOWING PANEL ERECTION.

- IT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, SUPPLY, AND INSTALLATION OF A TEMPORARY FALSEWORK SUPPORT SYSTEM TO ADEQUATELY BRACE THE ASSEMBLED PRECAST WALL UNITS UNTIL THE CONCRETE FOOTING HAS BEEN POURED AND ADEQUATELY CURED ACCORDING TO THE REINFORCED EARTH COMPANY SPECIFICATIONS, PLANS FOR THE TEMPORARY FALSEWORK SUPPORT SYSTEM SHOWING DIMENSIONS, SUPPORT POINTS, MEMBER SIZES, CONNECTIONS AND MATERIAL SPECIFICATIONS SHALL BE SUBMITTED TO THE REINFORCED EARTH COMPANY PRIOR TO WALL ERECTION. NOTWITHSTANDING ITS' REVIEW OF THE TEMPORARY FALSEWORK SUPPORT SYSTEM, THE REINFORCED EARTH COMPANY SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE OR LOSS CAUSED BY ANY DEFECT IN THE DESIGN AND/OR CONSTRUCTION OF THE TEMPORARY FALSEWORK SUPPORT SYSTEM. THRUST BLOCKS OR REACTION ASSEMBLIES SHALL BE OF SUFFICIENT SIZE SO THAT THE APPLIED SOIL PRESSURE DOES NOT EXCEED THE ALLOWABLE SOIL PRESSURE OR PRODUCE DETRIMENTAL DEFORMATIONS IN THE RESULTING POSITIONING OF THE ASSEMBLED PRECAST WALL UNITS.
- 18. CONCRETE COVER
 - CAST-IN-PLACE
 - 4" CLEAR ON REBAR FOR CONCRETE CAST AGAINST EARTH.
 3" CLEAR ON REBAR FOR ALL OTHER C.I.P. CONCRETE UNLESS NOTED
 OTHERWISE.
 - PRECAST CONCRETE
 - ALL REBARS IN PRECAST CONCRETE SHALL HAVE 2" MINIMUM CONCRETE COVER.
- 19. CONCRETE FOR PRECAST PANELS WILL BE PROVIDED BY THE REINFORCED EARTH COMPANY'S MANUFACTURING PLANT IN ACCORDANCE WITH SECTION 346 OF THE FLORIDA DOT SPECIFICATIONS.
- 20. THE REINFORCED EARTH COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.
- 21. THESE DRAWINGS ARE CERTIFIED WITH RESPECT TO THE INTERNAL STABILITY OF REINFORCED EARTH STRUCTURES ONLY
- 22. THIS DRAWING CONTAINS INFORMATION PROPRIETARY TO THE REINFORCED EARTH COMPANY, AND IS BEING FURNISHED FOR THE USE IN CONNECTION WITH FDOT PROJECTS ONLY, AND THE INFORMATION CONTAINED HEREIN IS NOT TO BE TRANSMITTED TO ANY OTHER ORGANIZATION UNLESS SPECIFICALLY AUTHORIZED IN WRITTING BY THE REINFORCED EARTH COMPANY. THE REINFORCED EARTH COMPANY IS EXCLUSIVE LICENSEE IN THE UNITED STATES UNDER PATENTS ISSUED TO HENRY VIDAL, AND THE FURNISHING OF THIS DRAWING DOES NOT CONSTITUTE AN EXPRESSED OR IMPLIED LICENSE UNDER THE VIDAL PATENTS.

THIS SYSTEM SHALL NOT BE USED IN ACUTE ANGLE SMALLER THAN 60°

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

TECHWALL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM REINFORCED EARTH COMPANY TECHWALL

Designed By

Designed By

Drawn By

Checked By

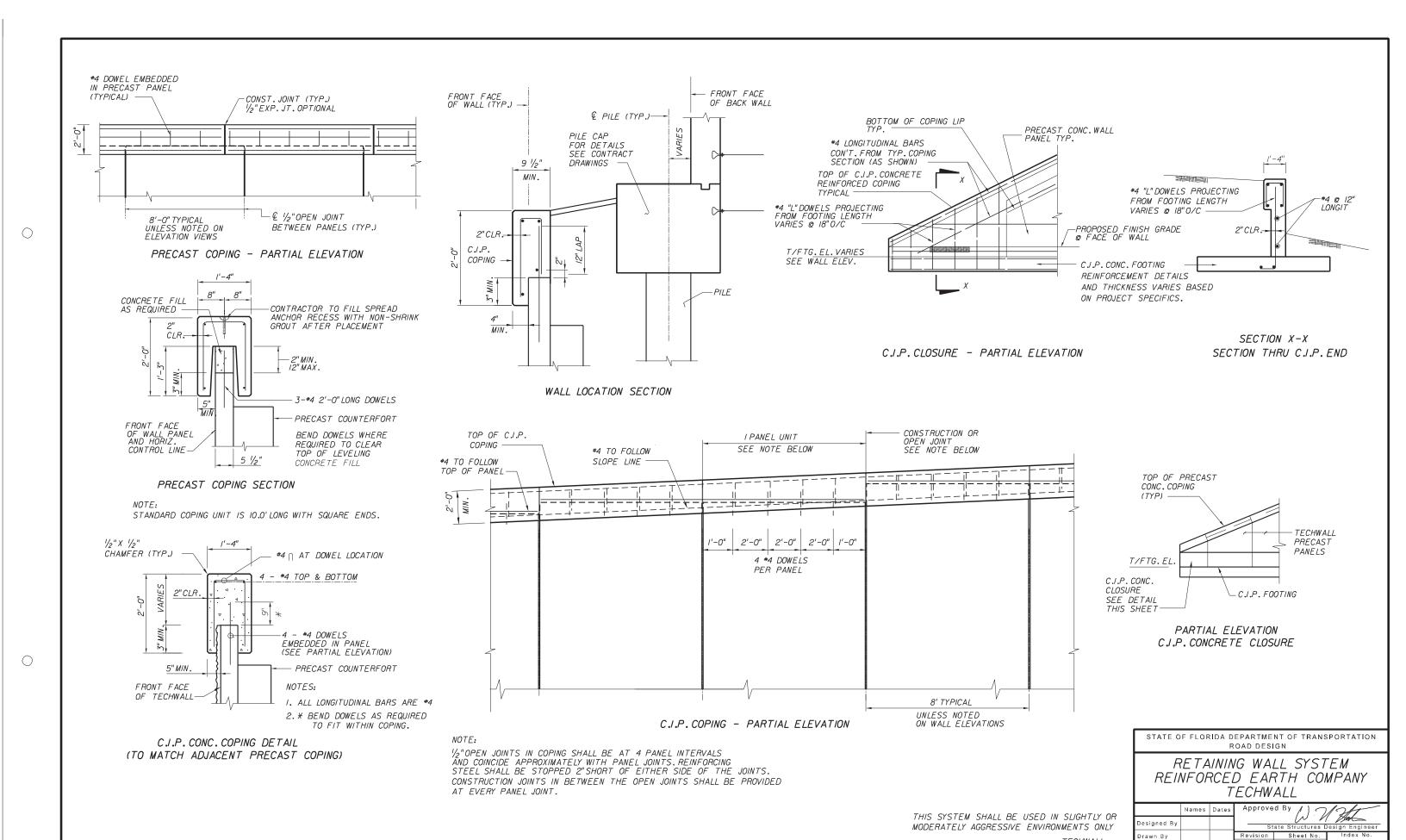
Names Dates Approved By

State Structures Design Engineer

Revision Sheet No. Index No.

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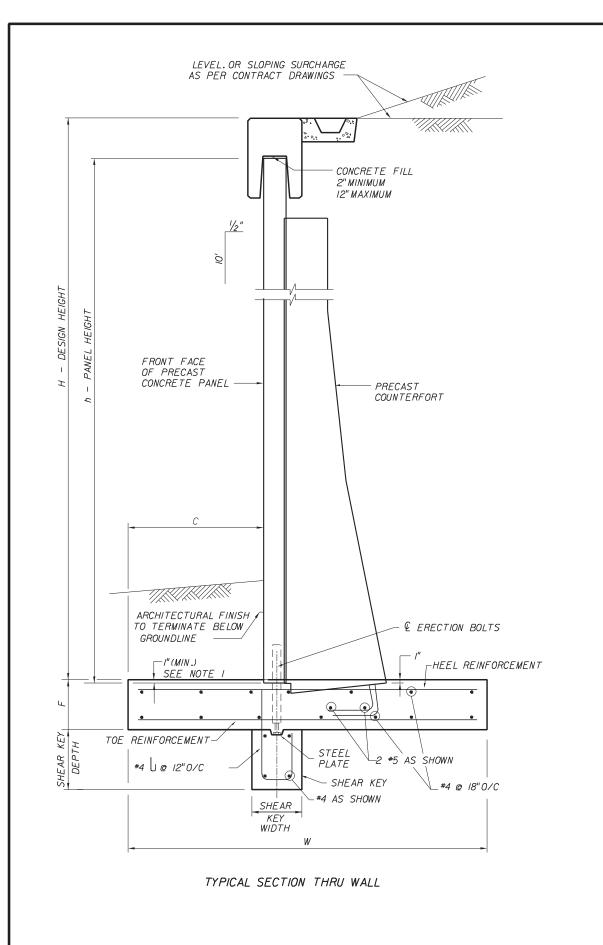
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TECHWALL

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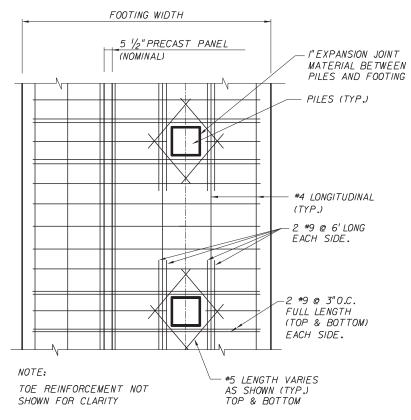
PILE SPACING (VARIES)

— © PILE (TYP.)

4'-0"

4'-0"

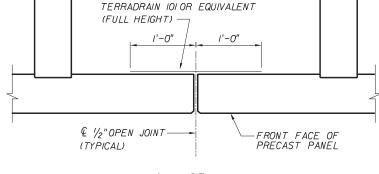
LAYOUT PRECAST PANEL W/COUNTERFORTS
RELATED TO PILE LAYOUT



PLAN-FOOTING AT ABUTMENT PILES

C,F,H,W AND THE REINFORCEMENT DETAILS ARE DETERMINED BY PROJECT SPECIFICS.

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY TECHWALL



USE GEOCOMPOSITE

JOINT DETAIL

NOTEC.

- I. THE BOTTOM EDGE OF THE ASSEMBLED PRECAST PANEL SHALL BE COVERED BY I"MINIMUM OF CAST-IN-PLACE FOOTING CONCRETE.
- 2. PRECAST WALL UNITS SHALL BE INSTALLED AT BATTER OF 1/2" PER 10' UNLESS OTHERWISE SHOWN ON CONSTRUCTION DRAWINGS.
- 3. FOR PANEL HEIGHTS OF 6.0' OR LESS COUNTERFORTS ARE NOT REQUIRED. PANELS WITHOUT COUNTERFORTS SHALL BE 8" THICK (NOMINAL). DETAILS WILL BE SHOWN ON CASTING DRAWINGS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM REINFORCED EARTH COMPANY TECHWALL

Designed By

Drawn By

Checked By

Dates

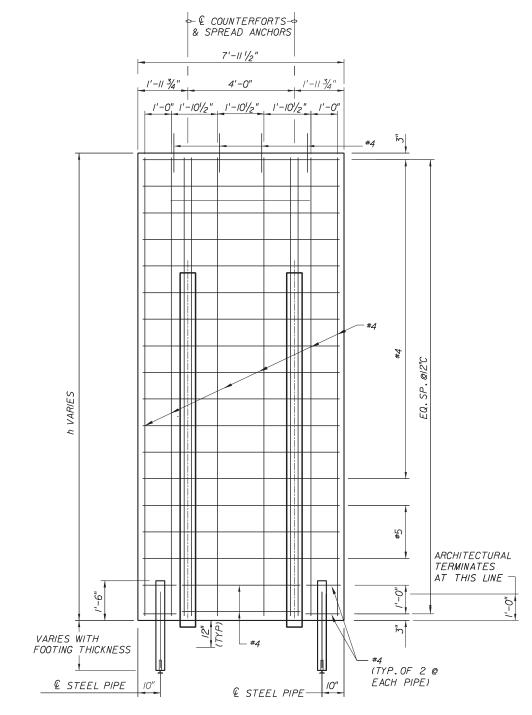
Approved By

State Structures Design Engineer

Revision Sheet No. Index No.

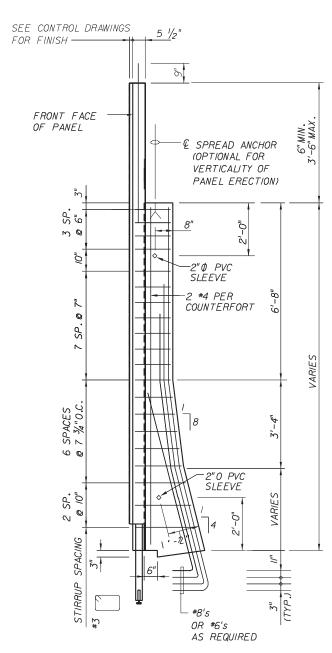
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PANEL ELEVATION

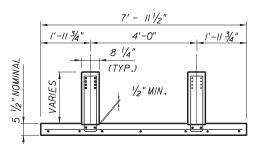
(REINFORCEMENT DETAILS MAY VARY WITH PROJECT SPECIFICS.)



COUNTERFORT - SIDE ELEVATION

(REINFORCEMENT DETAILS MAY VARY WITH PROJECT SPECIFICS.)

LIST OF MATERIALS						
CONCRETE: PANEL FACING (CY)	VARIES					
COUNTERFORT, EACH (CY)	VARIES					
TOTAL (CY)	VARIES					
TOTAL PANEL WT.(LB)	VARIES					
2"I.D. X I'-O"PVC SLEEVE	4					
SPREAD ANCHORS	2					



PANEL SECTION

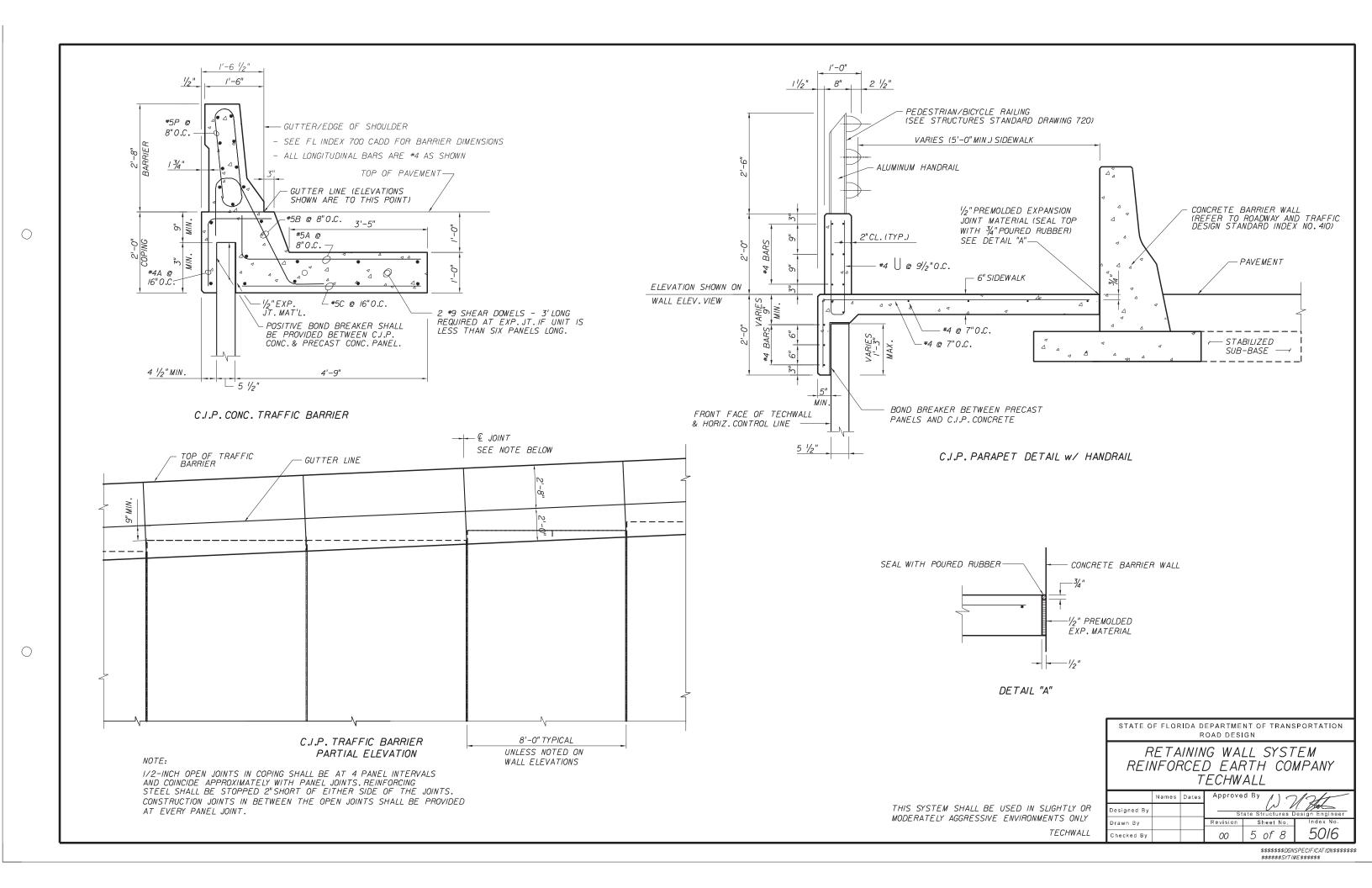
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

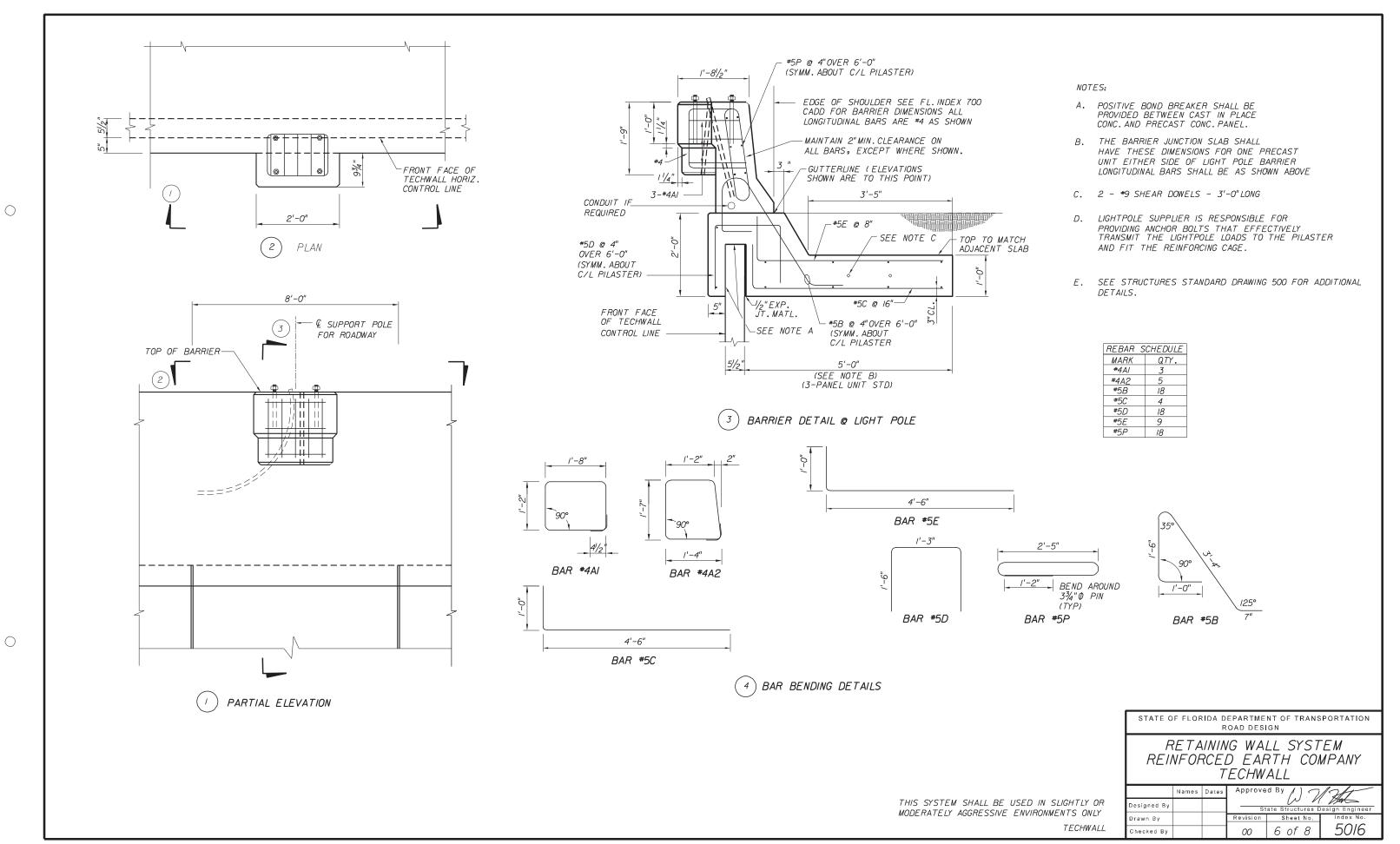
RETAINING WALL SYSTEM REINFORCED EARTH COMPANY TECHWALL

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

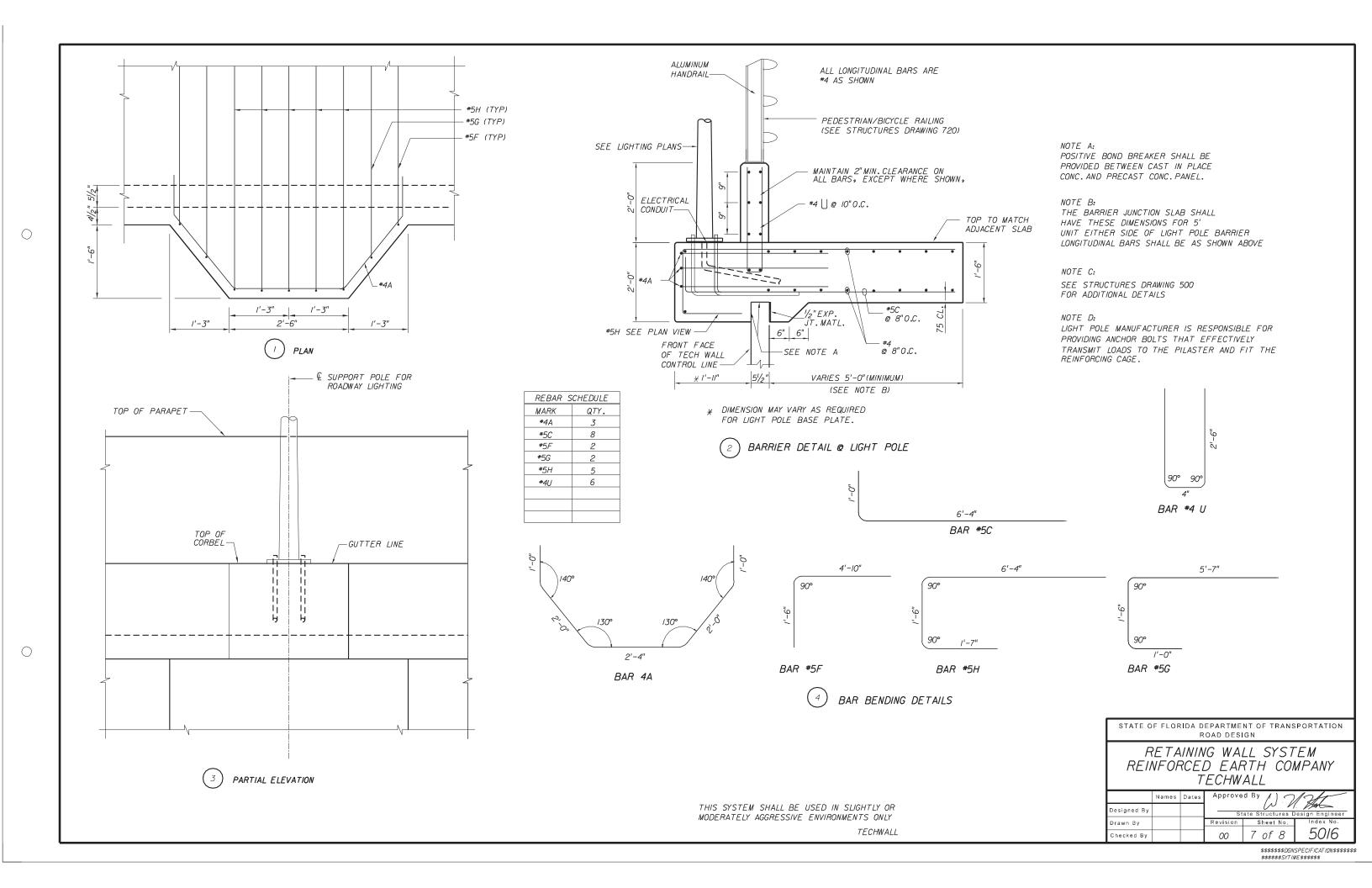
TECHWALL

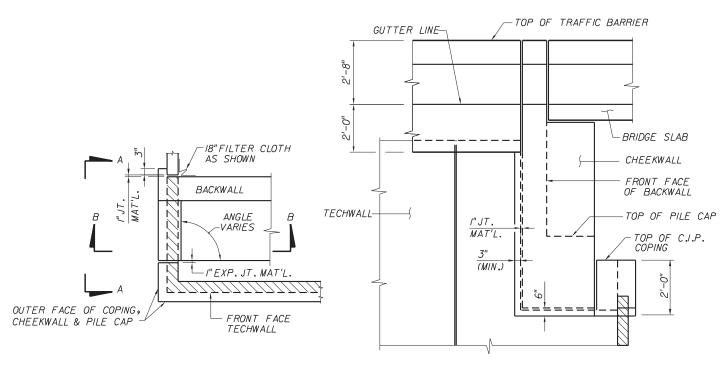
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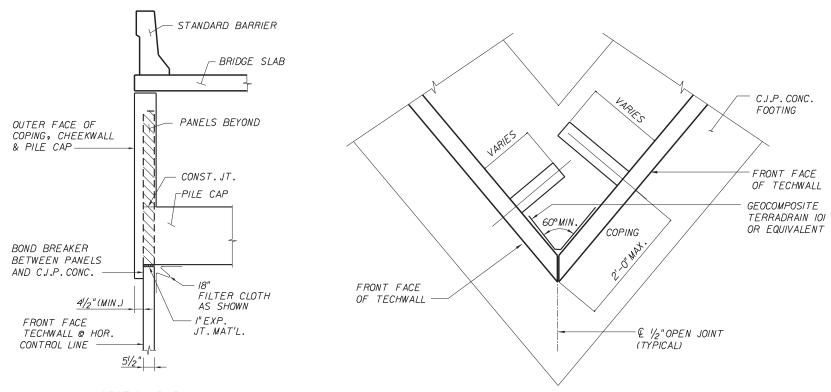
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PLAN VIEW @ BEND (TYP.)

SECTION A-A



SECTION B-B

ACUTE CORNER DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM REINFORCED EARTH COMPANY TECHWALL

Names Dates Approved By / Designed By rawn By Checked By 00 8 of 8

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY TECHWALL



HILFIKER MSE SQUARE PANEL WALL SYSTEM



GENERAL NOTES

DESIGN CRITERIA

- I. THE ATTACHED DETAILS ARE BASED ON THE ASSUMPTIONS THAT THE MATERIAL WITHIN THE REINFORCED VOLUME, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED COMPONENTS MEET THE GOVERNING AGENCIES SPECIFICATION FOR MECHANICALLY STABILIZED EARTH STRUCTURES
- 2. MINIMUM DESIGN PARAMETERS

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UNITIZED AT THE SITE. THE VALUE OF THE INTERNAL FRICTION ANGEL, PHI, THE COHESION, C, AND THE UNIT WEIGHT, GAMMA, SHALL BE PROVIDED IN THE SHOP DRAWINGS.

EXTERNAL STABILITY

OVERTURNING \$\, 2.00
\$\$LIDING \$\, 2.15
BEARING PRESSURE \$\, 2.5\$

OVERALL \$\$STABILITY\$\, 2.15

INTERNAL STABILITY

PULLOUT
STEEL YIELD STRESS = 0.47 F y

SERVICE LIFE = 75 YEARS

LIVE LOAD SURCHARGE = 250 PSF

- 3. THE MAXIMUM APPLIED BEARING PRESSURE AT THE INTERFACE OF THE FOUNDATION AND SELECT BACKFILL MATERIAL IS SHOWN ON THE PLANS. THE BEARING PRESSURE SHOWN IS THE MAXIMUM FOR THE GIVEN BASE MAT LENGTH, IT IS THE RESPONSIBILITY OF OTHERS TO DETERMINE THAT THE BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 4. ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED VOLUME AS DETERMINED BY THE ENGINEER SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL AS DIRECTED BY THE ENGINEER.
- 5. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY OTHERS. ON THE BASIS OF THIS INFORMATION, T&B STRUCTURAL SYSTEMS IS RESPONSIBLE FOR THE INTERNAL STABILITY OF THE STRUCTURE. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

WALL CONSTRUCTION

- I, WALLS FOUNDED ON CURVES SHALL HAVE THEIR PANELS DIMENSIONED AS A SERIES OF CORDS (AS DIMENSIONED IN SHOP DRAWINGS) IN ORDER TO MATCH THE REQUIRED WALL RADIUS.
- 2. FOR LOCATION AND ALIGNMENT OF THE MSE STRUCTURES REFERENCE THE RETAINING WALL CONTROL PLANS.
- 3. IF MANHOLE AND DROP INLETS ARE REQUIRED, THEY SHALL BE LOCATED AS SHOWN ON THE RETAINING WALL ELEVATION DRAWINGS.
- 4. IF PILES ARE LOCATED WITHIN THE REINFORCED VOLUME THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE WALL UNLESS AN ALTERNATE METHOD IS USED TO ISOLATE THE COLUMNS FROM THE REINFORCED VOLUME AS APPROVED BY THE ENGINEER.
- 5. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WE WITH SECTION 548 TO A LEVEL 2" (PLUS OR MINUS) ABOVE THE ELEVATION OF THE SOIL REINFORCING ELEMENT. NO SOIL REINFORCEMENT SHALL BE ATTACHED TO ANY PANEL BEFORE THE BACKFILL IS PLACED AT THE REQUIRED ELEVATION AND IS COMPACTED.
- 6. STRUCTURES GREATER THAN 20 FEET SHALL HAVE THE FINISHED GRADE PLACED AND COMPACTED AT THE FRONT FACE OF THE STRUCTURE BEFORE THE STRUCTURE HEIGHT EXCEEDS 20 FEET. FINISH GRADE SHALL BE COMPACTED TO 95 % OF AASHTO T-180 UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

- 7. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY GUARDRAIL POSTS PRIOR TO PLACING THE TOP ROW OF SOIL REINFORCEMENT. THE POST SPACING SHALL BE ADJUSTED TO AVOID CONFLICTS WITH THE LONGITUDINAL SOIL REINFORCING WIRE. CUTTING OF THE LONGITUDINAL WIRE SHALL BE ALLOWED ONLY AS DIRECTED BY THE ENGINEER.
- 8. IF EXISTING OR FUTURE STRUCTURES ARE TO BE PLACED IN THE REINFORCED VOLUME THAT INTERFERE WITH THE PROPER PLACEMENT OF THE SOIL REINFORCEMENT THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY FOR A COURSE OF ACTION.
- 9. TOP COPING PANELS BENEATH CAST-IN-PLACE COPING SHALL HAVE 1/2* DOWELS PROTRUDING FROM THEIR TOP EDGE.
- 10. FOR OTHER INFORMATION PERTAINING TO THE CONSTRUCTION OF THE HILFIKER RETAINING WALL PLEASE REFER TO T&B STRUCTURAL SYSTEMS ERECTION MANUAL.
- II. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DEFLECT THE TOP ROW OF SOIL REINFORCEMENT DOWNWARD SO AS TO NOT CONFLICT WITH ROADWAY MIXING OPERATIONS AND/OR ROADWAY CONSTRUCTION OPERATIONS. ANY SOIL REINFORCING MATERIAL THAT IS DAMAGED SHALL BE REPLACED AT THE CONTRACTORS EXPENSE.

MISCELLANEOUS NOTES

I. NOMINAL SOIL REINFORCING GRID LENGTH

THE WELDED WIRE MESH IS MANUFACTURED IN LENGTHS CORRESPONDING TO THE DIMENSION "B" AS GIVEN IN THE RETAINING WALL ELEVATIONS. THE ACTUAL LENGTH FROM THE FRONT FACE OF THE PANEL TO THE TAIL OF THE SOIL REINFORCING GRID IS PLUS 12" THIS ACCOUNTS FOR THE THICKNESS OF THE PANEL AND THE LOCATION OF THE CONNECTION OF THE SOIL REINFORCING MAT WITH THE PANEL ANCHOR. THE FOUNDATION SHALL BE EXCAVATED TO AN EXTENT OF "B" PLUS 12".

2. SELECT BACKFILL QUANTITY

THE REQUIRED VOLUME OF IN-PLACE SELECT BACKFILL IS CALCULATED BY MULTIPLYING THE RETAINING WALL FACE AREA BY THE SOIL REINFORCING LENGTH. THIS IS PERFORMED AT EACH INDIVIDUAL SEGMENT OF WALL FOR EACH CORRESPONDING "B". THE BACKFILL QUANTITY IF GIVEN BY T&B STRUCTURAL SYSTEMS IS AN ESTIMATE ONLY. THE CONTRACTOR IS ULTIMATELY TO DETERMINE THE QUANTITY OF SELECT BACKFILL MATERIAL THAT IS REQUIRED.

3. PANEL FINISH

THE CONCRETE PANELS SHALL HAVE A PLAIN STEEL FORM FINISH UNLESS OTHERWISE SPECIFIED ON THE RETAINING WALL CONTROL PLANS.

- 1. THE FOLLOWING MATERIALS ARE SUPPLIED BY T&B STRUCTURAL SYSTEMS
 - PRECAST CONCRETE FACING PANEL
 - SOIL REINFORCING GRIDS
 - · CONNECTION PINS
 - 1/2" DIAMETER ALIGNMENT PINS • 60 DURO 3/4" X 8" BEARING PADS
 - * SYNTHETIC INDUSTRIES GEOTEX 401 NONWOVEN GEOTEXTILE FILTER FABRIC

ANY OTHER MATERIAL REQUIRED TO BUILD THE MSE STRUCTURES ACCORDING TO THE GOVERNING SPECIFICATION SHALL BE SUPPLIED BY THE CONTRACTOR.

5. T&B STRUCTURAL SYSTEM SUPPLIES MECHANICALLY STABILIZED EARTH STRUCTURAL COMPONENTS FOR USE WITH THE HILFIKER RETAINING WALL SYSTEMS FOR THE STRUCTURES DETAILED HEREIN. THE ERECTION MANUAL PROVIDED BY T&B STRUCTURAL SYSTEMS IS A GENERAL GUIDELINE FOR ERECTING THE HILFIKER RETAINING WALL SYSTEM. ALL OUALITY CONTROL PROCEDURES, STAGING PROCEDURES, MATERIAL HANDLING, AND SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. THIS DOES NOT RELIEVE THE CONTRACTOR OF THE OBLIGATION TO CONSTRUCT THE RETAINING WALL ACCORDING TO THE PROJECT PLANS AND SPECIFICATIONS AND ALL LAWS OF THE GOVERNING STATE.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

THIS SYSTEM FOR USE IN MODERATELY OR SLIGHT AGGRESSIVE ENVIRONMENTS ONLY

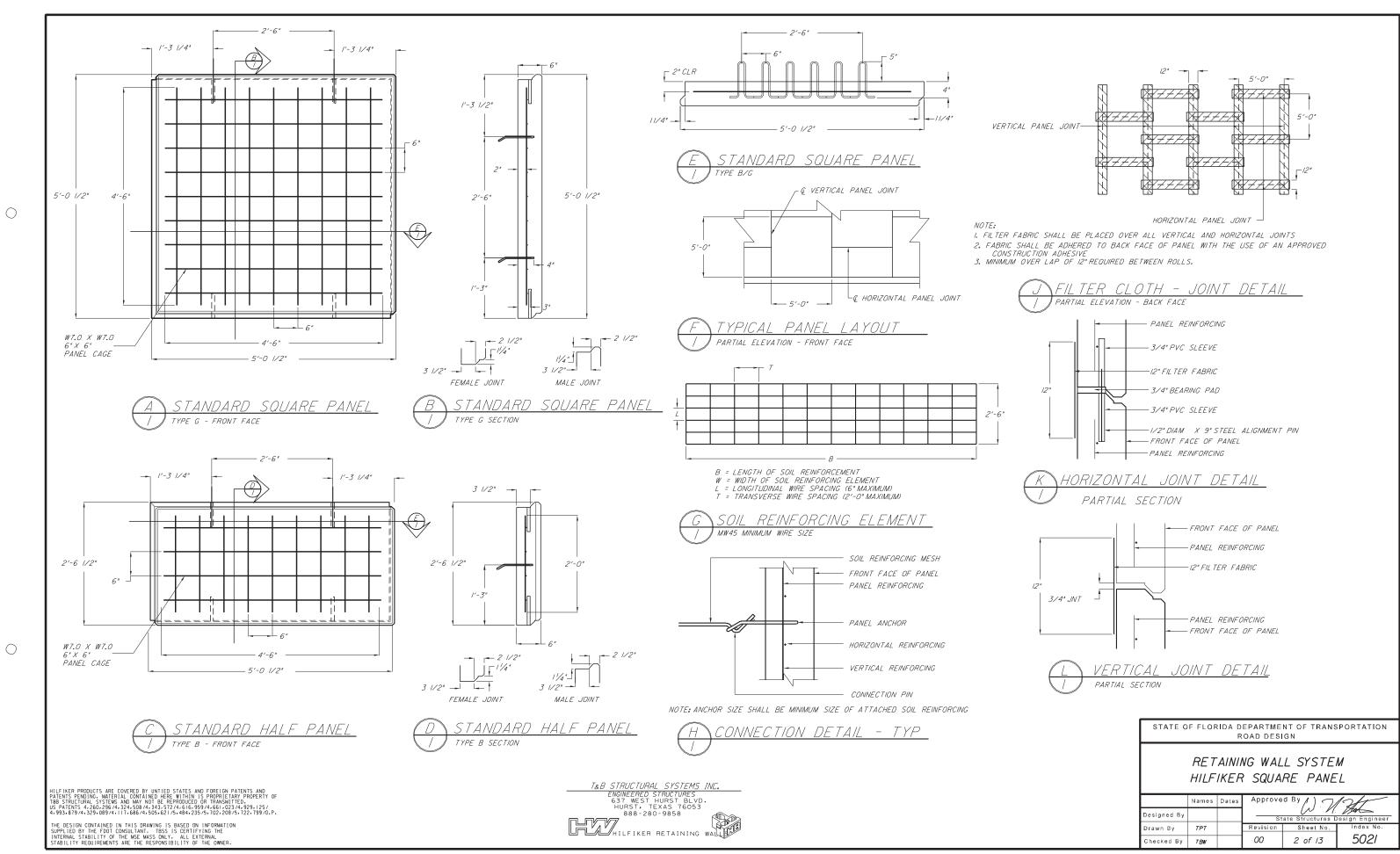
RETAINING WALL SYSTEM HILFIKER SQUARE PANEL

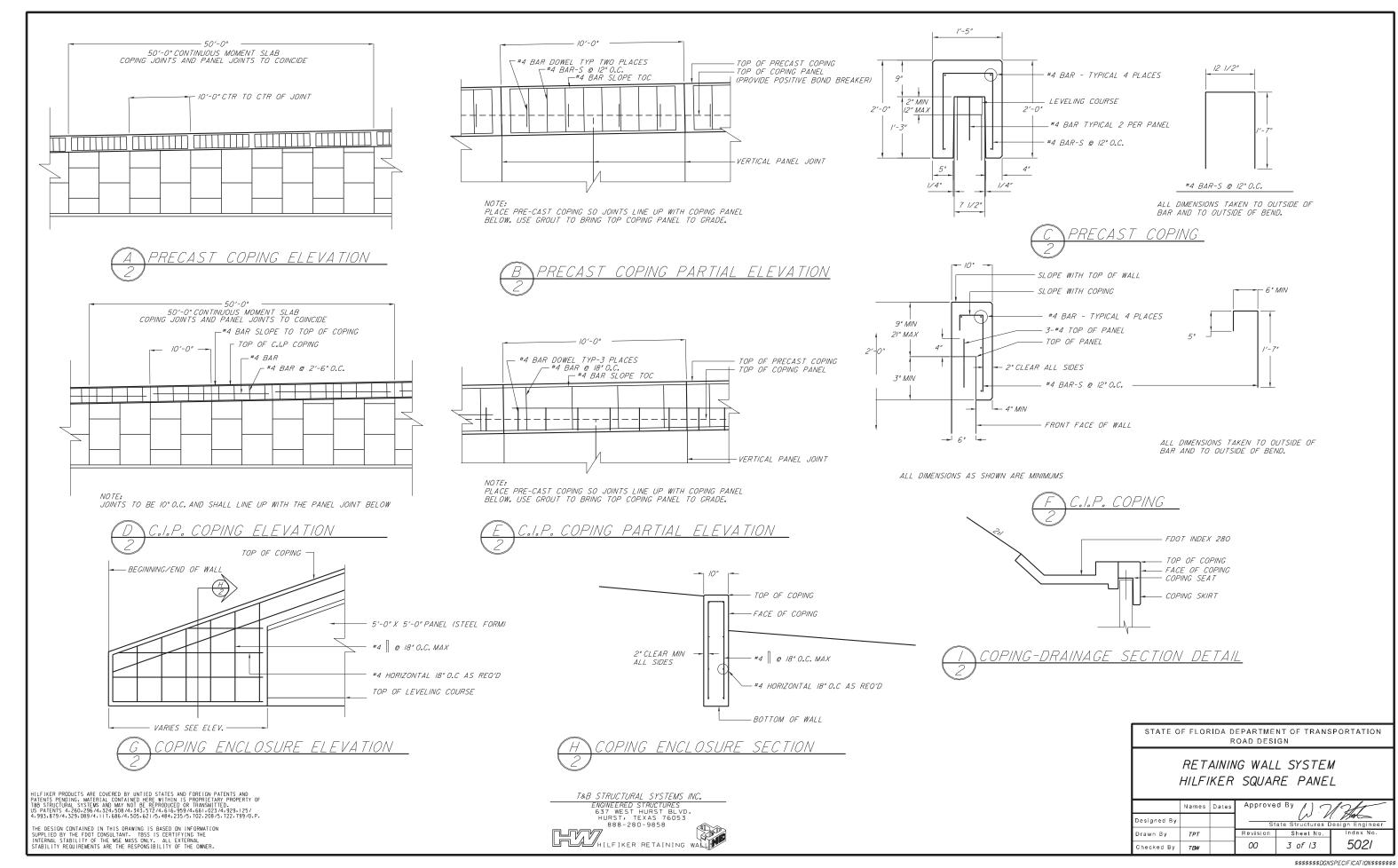
	Names	Dates	Approve	d By /) ?	12/1	
signed By			State Structures Design Engineer			
awn By	TPT		Revision	Sheet No.	Index No.	
ecked By	T.BW		00	1 of 13	<i>5021</i>	

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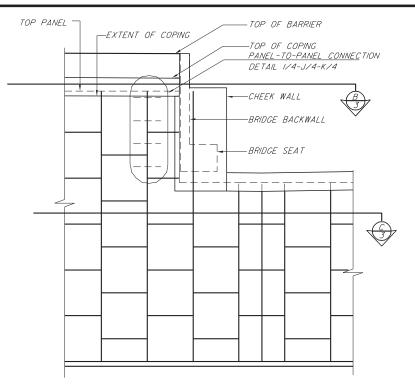
THE DESIGN CONTAINED IN THIS DRAWING IS BASED ON INFORMATION SUPPLIED BY THE FDDT CONSULTANT. TBSS IS CERTIFYING THE INTERNAL STABILITY OF THE MSE MASS ONLY. ALL EXTERNAL STABILITY REQUIREMENTS ARE THE RESPONSIBILITY OF THE OWNER.

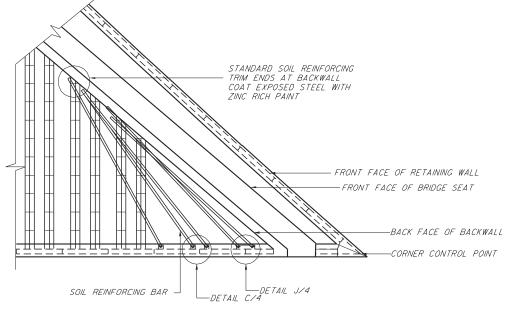






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ABUTMENT RETAINING WALL SOIL REINFORCEMENT NOT SHOWN FOR CLARITY END BENT BACK WALL REINFORCING NOT SHOWN FOR CLARITY

CUTE CORNER PLAN SECTION

STANDARD SOIL REINFORCING TRIM ENDS AT BACKWALL COAT EXPOSED STEEL WITH ABUTMENT SOIL REINFORCEMENT DETAIL C/4/-D/4 ABUTMENT SOIL REINFORCING CORNER CONTROL POINT ABUTMENT SOIL REINFORCING SOIL REINFORCING BAR STANDARD SOIL REINFORCING STANDARD SOIL REINFORCING TRIM ENDS AT BACKWALL COAT EXPOSED STEEL WITH

- FRONT FACE OF RETAINING WALL

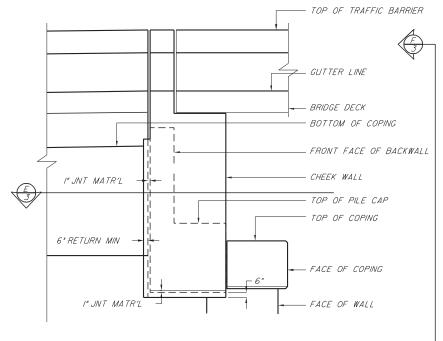
NOTE: REFERENCE DETAIL G/5 FOR ABUTMENT SOIL REINFORCEMENT SHOWN

C ACUTE CORNER PLAN SECTION 3



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EVATION AT CHEEK WALL

FACE OF PILE CAP

FRONT FACE OF BACKWALL

EXTENT OF BACKWALL

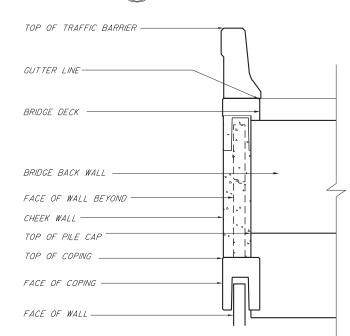
FACE OF COPING

EXTENT OF COPING

CHEEK WALL

FACE OF WALL

FACE OF WALL



ZINC RICH PAINT

__STANDARD SOIL REINFORCING

E PLAN SECTION AT CHEEK WALL

F SECTION AT CHEEK WALL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM HILFIKER SQUARE PANEL

	Names	Dates	Approve	d By /) 7	12/	
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
Checked By	TBW		00	4 of 13	5021	

LLFIKER PRODUCTS ARE COVERED BY UNTIED STATES AND FOREIGN PATENTS AND TATENTS PENDING. MATERIAL CONTAINED HERE WITHIN IS PROPRIETARY PROPERTY OF 3B STRUCTURAL SYSTEMS AND MAY NOT BE REPRODUCED OR TRANSMITTED. 5 PATENTS 4,260,296/4,324.508/4.343.4525/5/2/4.666.959/4.661.023/4,929,125/ .933.879/4.329.089/4.117,686/4,505.621/5,484.235/5/T02.208/5.722.799/0.P.

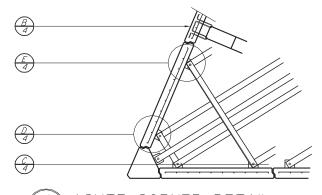
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TAB STRUCTURAL SYSTEMS INC.

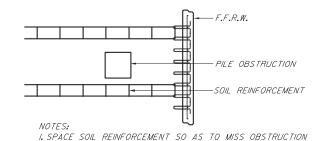
ENGINEERED STRUCTURES
637 WEST HURST BLVD.
HURST, TEXAS 76053
888-280-9858

HILFIKER RETAINING WALL

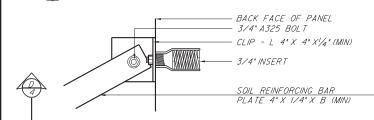
\$\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$\$ \$\$\$\$\$\$SYTIME\$\$\$\$\$\$



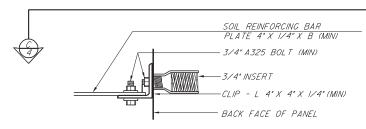
CORNER DETAIL ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.



`ONTINUOUS ANCHOR PLAN ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.



REINFORCING BAR PLAN

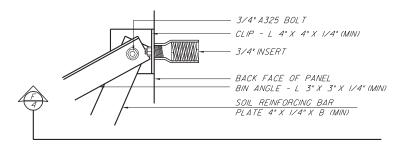


REINFORCING BAR DETAIL ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

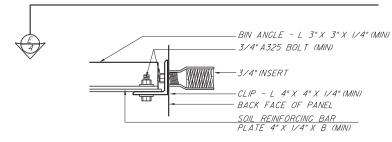
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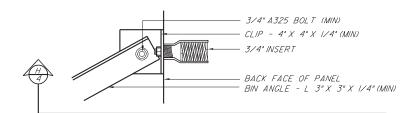
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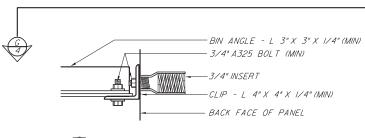
COMBINATION ANGLE/BAR PLAN ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.



OMBINATION STRAP/BAR DETAIL ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

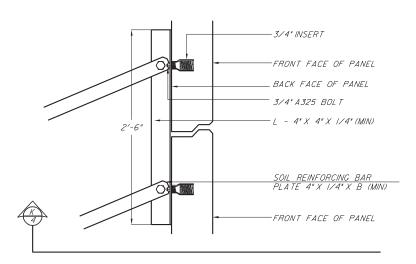


BIN CLIP PLAN DETAIL ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

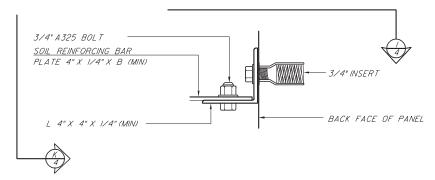


CLIP <u>SECTION DETAIL</u> ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

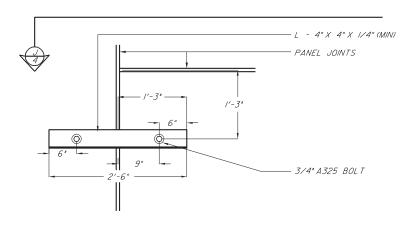
T&B STRUCTURAL SYSTEMS INC. ENGINEERED STRUCTURES
637 WEST HURST BLVD.
HURST, TEXAS 76053 888-280-9858 HILFIKER RETAINING WALL



PANEL-TO-PANEL CONNECTION PLAN ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.



PANEL-TO-PANEL CONNECTION SECTION ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

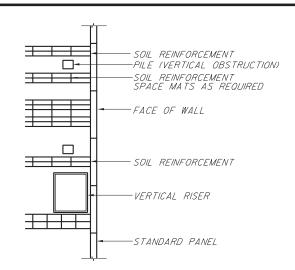


PANEL-TO-PANEL CONNECTION ELEVATION

ALL STEEL TO BE HOT DIP GALVANIZED U.N.O.

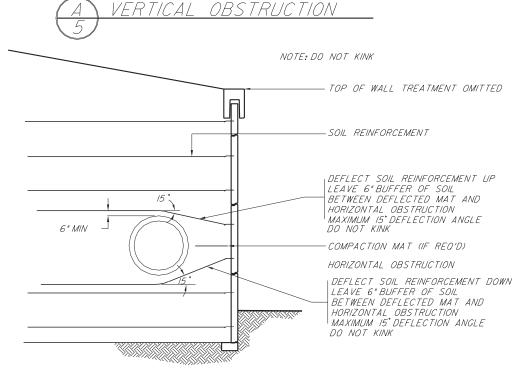
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

	Names	Dates	Approve	d By /)	12/1	
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
Checked By	TBW		00	5 of 13	502 <i>l</i>	



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- I. VERTICAL OBSTRUCTIONS REQUIRE SPECIAL DESIGN CONSIDERATIONS 2. THE DETAIL AS SHOWN IS FOR CONCEPT ONLY AND MAY VARY ON
- 3. REFERENCE SPECIAL DESIGN CALCULATIONS FOR DETAILS AND COMPONENT
- TYPE AND SIZE 4. OBSTRUCTION SHALL BE INSTALLED BEFORE WALL

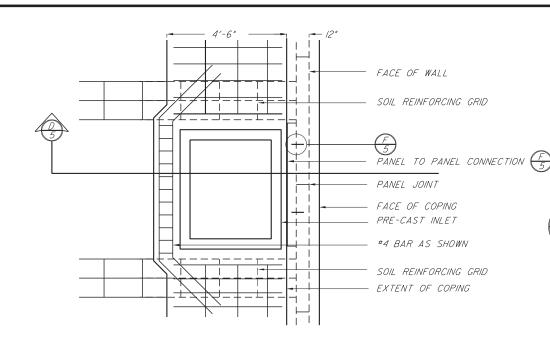


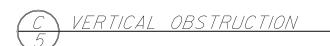
- I. HORIZONTAL OBSTRUCTIONS REQUIRE SPECIAL DESIGN CONSIDERATIONS 2. THE DETAIL AS SHOWN IS FOR CONCEPT ONLY AND MAY VARY ON
- FINAL DESIGN 3. REFERENCE SPECIAL DESIGN CALCULATIONS FOR DETAILS AND COMPONENT TYPE AND SIZE

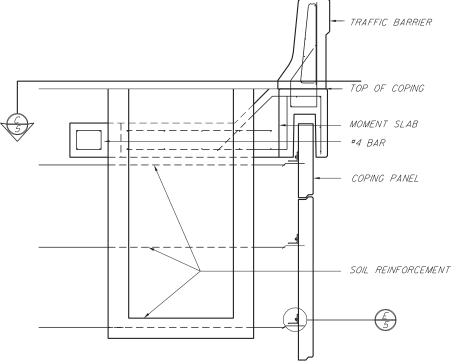


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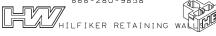


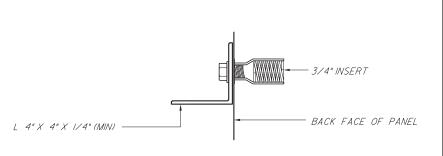




T&B STRUCTURAL SYSTEMS INC.

ENGINEERED STRUCUTRES
637 WEST HURST BLVD.
HURST, TEXAS 76053 888-280-9858

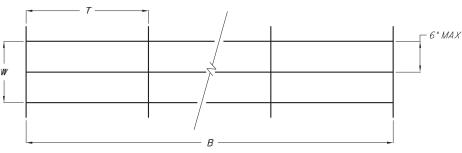




PANEL-TO-PANEL CONNECTION DETAIL

L 4" X 4" X 1/4" (MIN) 3/4" INSERT BACK FACE OF PANEL 3/4" A325 BOLT

PANEL-TO-PANEL CONNECTION DETAIL



B = SOIL REINFORCING LENGTH T = TRANSVERSE WIRE SPACING (2'-0" MAX) W = WIDTH OF SOIL REINFORCING ELEMENT

NOTE: THE MAT SHOWN IS USED TO PASS OBSTRUCTIONS AND TYPICALLY IS A WELDED WIRE MESH WITH LARGE DIAMETER WIRES. THE LONGITUDINAL WIRE SHALL BE EQUAL TO OR SMALLER THAN THE PANEL ANCHOR, A MINIMUM OF THREE LONGITUDINAL WIRES IS REQUIRED. THE MINIMUM WIRE SIZE SHALL

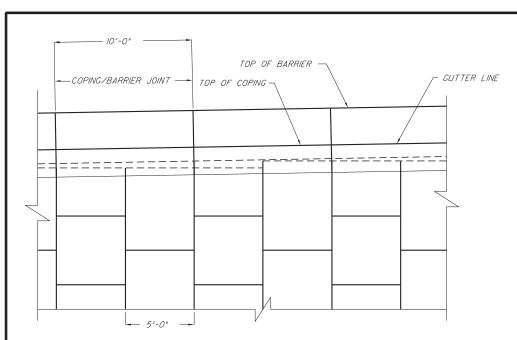
OBSTRUCTION SOIL REINFORCING PLAN

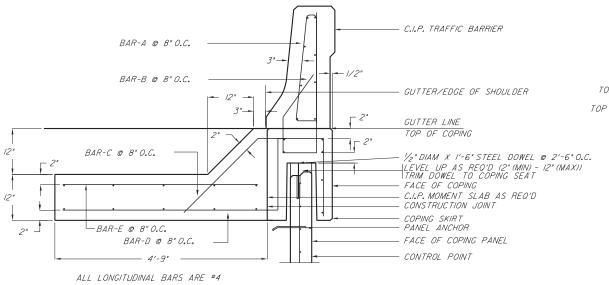
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

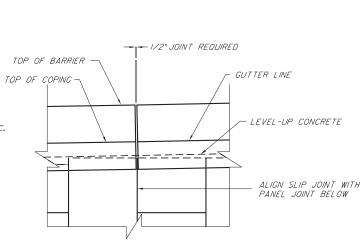
> RETAINING WALL SYSTEM HILFIKER SQUARE PANEL

	Names	Dates	Approved By			
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
Checked By	TBW		00	6 of 13	<i>502l</i>	

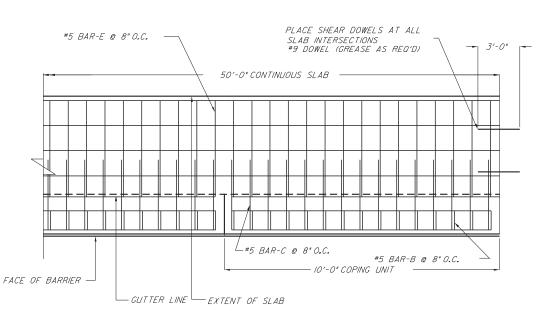
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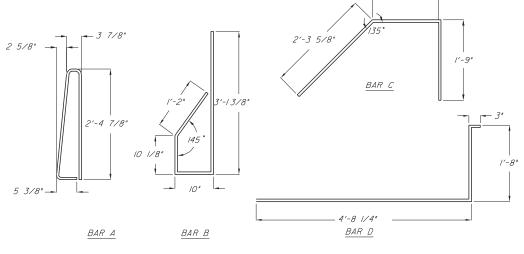


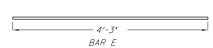


PRECAST COPING WITH C.I.P. BARRIER ELEVATION



PRECAST COPING WITH C.I.P. BARRIER AND C.I.P. JUNCTION SLAP

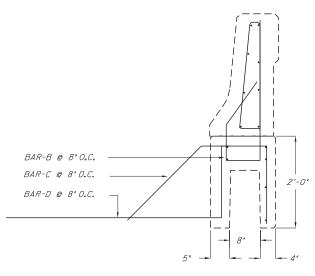




REBAR SCHEDULE							
MARK	SIZE	OTY	BENDING				
А	#5	//	AS DETAILED	AS DETAILED			
В	#5	//	AS DETAILED	AS DETAILED			
С	#5	//	AS DETAILED	AS DETAILED			
D	#5	//	AS DETAILED	AS DETAILED			

OUANTITIES SHOWN ARE FOR A 10'-0" COPING SECTION

TRAFFIC BARRIER SLIP JOINT



REFERNCE FDOT INDEX 700 FOR BARRIER DIMENSIONS NO SHOWN

PRECAST COPING REBAR LAYOUT

PRECAST COPING WITH C.I.P. BARRIER PLAN

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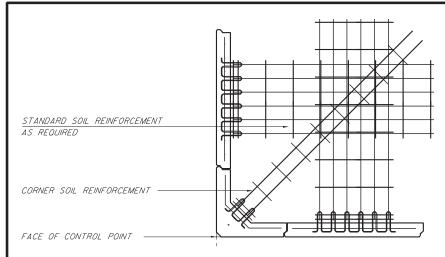
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PRECAST BARRIER/COPING REINFORCING

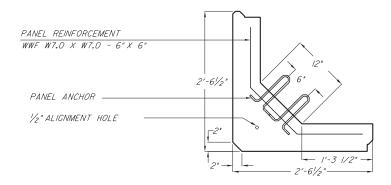
T&B STRUCTURAL SYSTEMS INC. ENGINEERED STRUCUTRES
637 WEST HURST BLVD.
HURST, TEXAS 76053
888-280-9858 HILFIKER RETAINING WALL STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

	Names	Dates	Approve	d By /) ?	12/1
Designed By			St	ate Structures E	Design Engineer
Drawn By	TPT		Revision	Sheet No.	Index No.
Checked By	TBW		00	7 of 13	5021

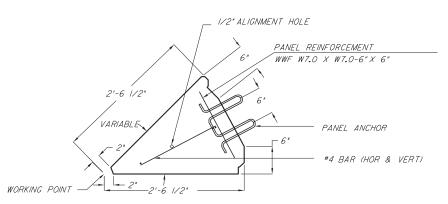


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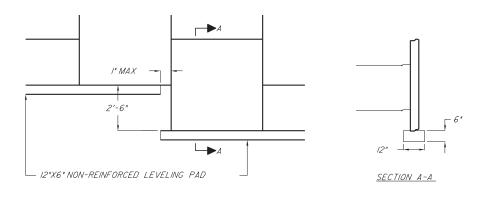
1*BTUSE CORNER PANEL* PANEL ANGLE VARIES FROM 90°TO 180





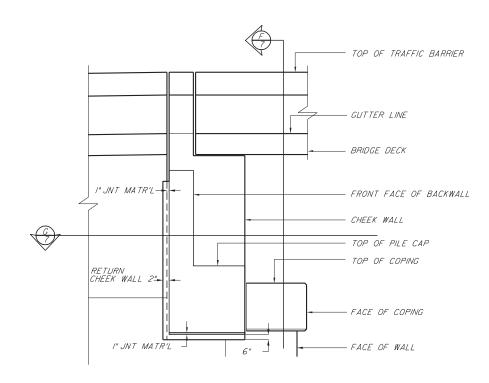
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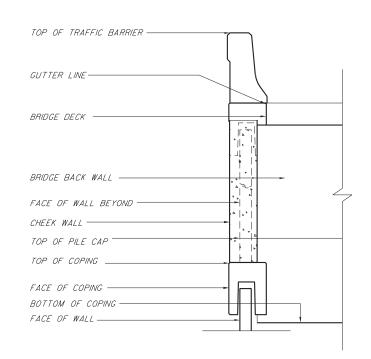
NOTE: LEVELING COURSE SHALL BE PLACED TO THE ELEVATIONS AS SHOWN ON THE PLANS. TOLERANCE FOR ELEVATIONS SHALL BE PLUS-MINUS 1/8"

EVELING COURSE STEP ELEVATION

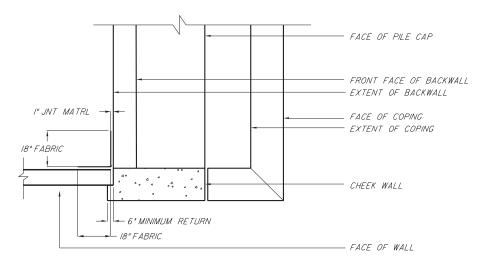






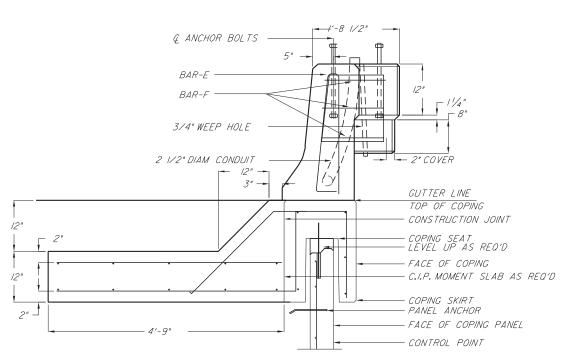






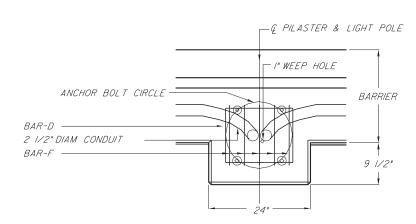
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

	Names	Dates	Approve	d By /) ?	12/
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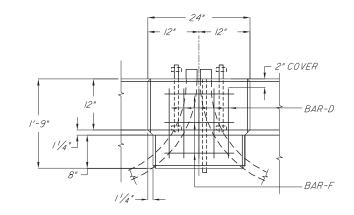
FOR ADDITIONAL DETAILS REFERENCE FDOT LIGHT POLE PILASTER SEE STRUCTURES STANDARD DRAWING 500 FOR JUNCTION SLAB DIMENSIONS AND REINFORCING REFERENCE SHEET HW-6

A PRECAST COPING WITH PILASTER SECTION 8



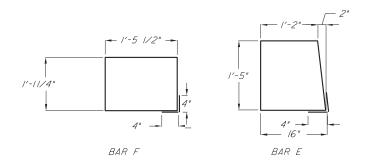
FOR ADDITIONAL DETAILS REFERENCE FDOT LIGHT POLE PILASTERS SEE STRUCTURES STANDARD DRAWING 500





FOR ADDITIONAL DETAILS REFERENCE FDOT LIGHT POLE PILASTER SEE STRUCTURES STANDARD DRAWING 500

C PILASTER ELEVATION 8



REBAR SCHEDULE

MARK	SIZE	OTY	LENGTH	BENDING
D	#5	5	AS DETAILED	AS DETAILED
F	#5	3	AS DETAILED	AS DETAILED

D PILASTER REINFORCING SCHEDULE

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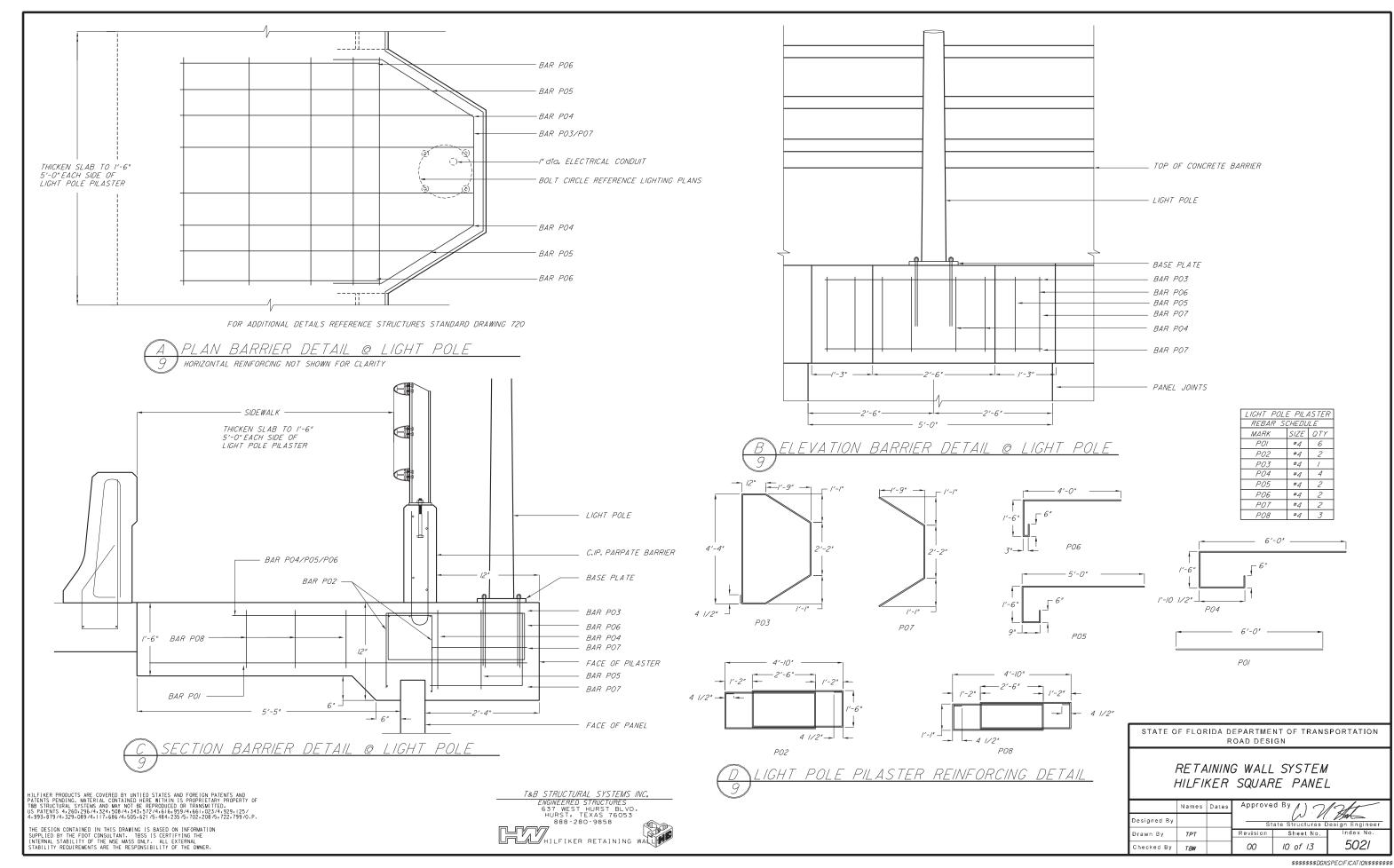
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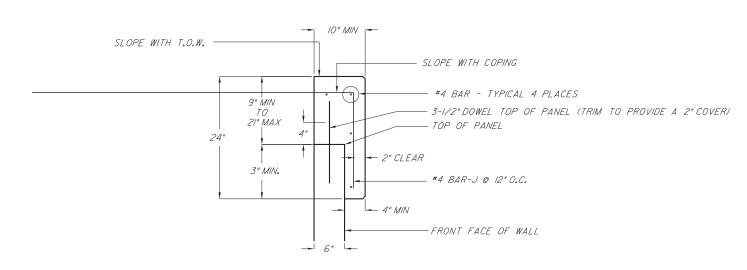
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

	Names	Dates	Approve	d By /) 2	12/	
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
Checked By	T BW		00	9 of 13	<i>5021</i>	

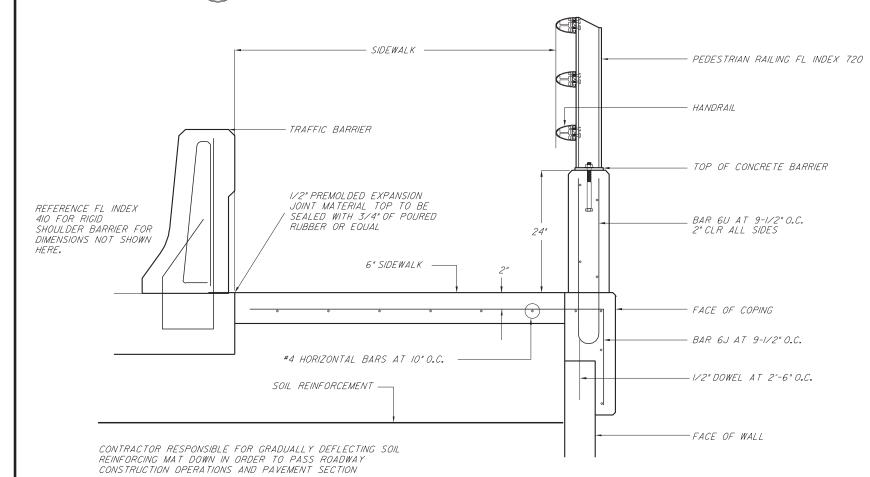


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A SECTION C.I.P. PARAPET COPING HORIZONTAL REINFORCING NOT SHOWN FOR CLARITY



SECTION C.I.P. BARRIER WITH PEDESTRIAN RAILING

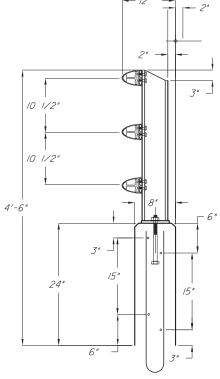
HILFIKER PRODUCTS ARE COVERED BY UNTIED STATES AND FOREIGN PATENTS AND PATENTS PENDING. MATERIAL CONTAINED HERE WITHIN IS PROPRIETARY PROPERTY OF TAB STRUCTURAL SYSTEMS AND MAY NOT BE REPRODUCED OR TRANSMITTED. US PATENTS 4.260.296/4.324.508/4.3434.572/4.616.959/4.661.023/4.929.125/4.993.879/4.329.089/4.117,686/4.505.621/5.484.235/5.702.208/5.722.799/0.P.

THE DESIGN CONTAINED IN THIS DRAWING IS BASED ON INFORMATION SUPPLIED BY THE FOOT CONSULTANT. TBSS IS CERTIFYING THE INTERNAL STABILITY OF THE MSE MASS ONLY. ALL EXTERNAL STABILITY REQUIREMENTS ARE THE RESPONSIBILITY OF THE OWNER.

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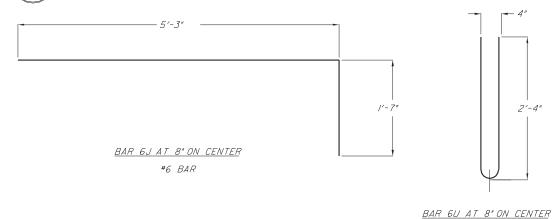
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SECTION C.I.P. PEDESTRIAN BARRIER

) REFERENCE STRUCTURES STANDARD DRAWING 720 FOR DETAILS NOT SHOWN

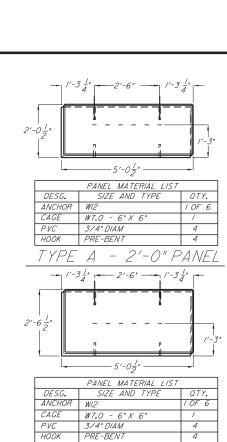


D C.I.P. COPING WITH PEDESTRIAN BARRIER BAR DETAILS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

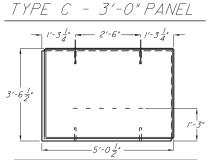
#6 BAR

	Names	Dates	Approve	d By /)	12/1
Designed By			St	ate Structures E	Design Engineer
Drawn By	TPT		Revision	Sheet No.	Index No.
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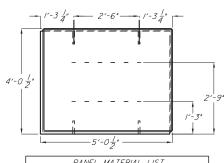


TYPE	B - 2'-6" PANEL
1'-3	2'-6" 1'-3
	7
3'-0 ½"	

-	5'-0 <u>'</u> 7"	-
	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	OTY.
ANCHOR	WI2	1 OF 6
CAGE	W7.0 - 6" X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4

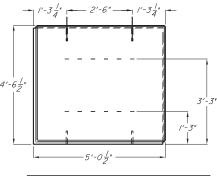


	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	OTY.
ANCHOR	WI2	1 OF 6
CAGE	W7.0 - 6"X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4



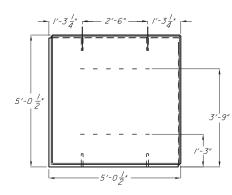
	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	QTY.
ANCHOR	WI2	2 OF 6
CAGE	W7.0 - 6" X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4

TYPE E - 4'-0" PANEL



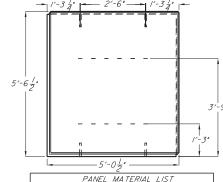
	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	OTY.
ANCHOR	WI2	2 OF 6
CAGE	W7.0 - 6" X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4

TYPE F - 4'-6" PANEL

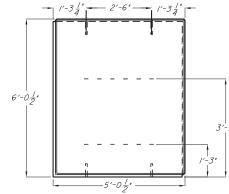


	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	QTY.
ANCHOR	WI2	2 OF 6
CAGE	W7.0 - 6" X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4

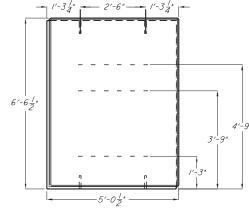
TYPE G - 5'-0" PANEL



		PANEL MATERIAL LIST	
	DESG.	SIZE AND TYPE	OTY.
	ANCHOR	WI2	2 OF 6
	CAGE	W7.0 - 6"X 6"	/
	PVC	3/4" DIAM	4
	HOOK	PRE-BENT	4
7	YPF	H - 5'-6"PA	NF/



	DESG.	SIZE AND TYPE	OTY.
	ANCHOR	WI2	2 OF 6
	CAGE	W7.0 - 6" X 6"	/
	PVC	3/4" DIAM	4
	HOOK	PRE-BENT	4
7	YPE	J - 6'-0" PA	NEL

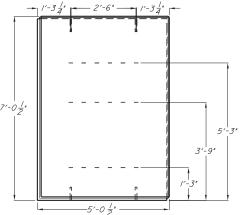


	PANEL MATERIAL LIST	
DESG.	SIZE AND TYPE	QTY.
ANCHOR	WI2	3 OF 6
CAGE	W7.0 - 6" X 6"	/
PVC	3/4" DIAM	4
HOOK	PRE-BENT	4
TYPE	K - 6'-6"PA	4NEL

T&B STRUCTURAL SYSTEMS INC.

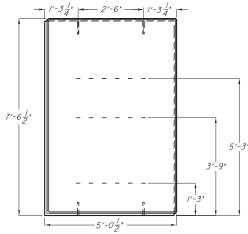
ENGINEERED STRUCTURES
637 WEST HURST BLVD.
HURST, TEXAS 76053
888-280-9858





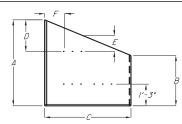
5'-0 2'							
PANEL MATERIAL LIST							
DESG. SIZE AND TYPE QTY.							
ANCHOR	W12	3 OF 6					
CAGE	W7.0 - 6" X 6"	/					
PVC	3/4" DIAM	4					
HOOK	PRE-BENT	4					

TYPE L - 7'-0" PANEL



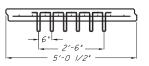
PANEL MATERIAL LIST						
DESG.	SIZE AND TYPE	QTY.				
ANCHOR	WI2	10F 6				
CAGE	W7.0 - 6" X 6"	/				
PVC	3/4" DIAM	4				
HOOK	PRE-BENT	4				

TYPE M - 7'-6" PANEL

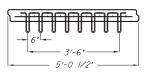


DESG.	SIZE AND TYPE	QTY.				
ANCHOR	W12	VARIES				
CAGE	W7.0 - 6" X 6"	/				
PVC	3/4" DIAM	4				
HOOK	PRE-BENT	4				

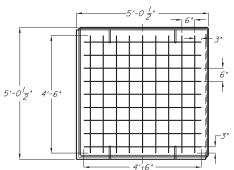
SPECIAL SLOPED PANEL



STANDARD ANCHOR LAYOUT



CONTINUOUS ANCHOR LAYOUT



CENTERLINE OF FORM - 4' 6"

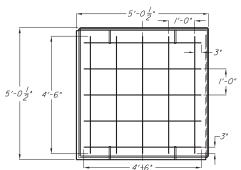
NOTE:

NOTE: 1. MINIMUM 3° COVER AT ALL EDGES 2. CENTER MESH IN FORM 3. WIRE MESH TO BE PLACED ON TOP OF PVC ALIGNMENT

SLEEVES.
4. TRIM AS REQUIRED
5. MINIMUM W7.0 X W7.0 WWF

PANEL REINFORCING LAYOUT

WELDED WIRE MESH



CENTERLINE OF FORM

NOTE:

1. MINIMUM 3" COVER AT ALL EDGES

2. CENTER REBAR IN FORM

3. REBAR TO BE PLACED ON TOP OF PVC ALIGNMENT
SLEEVES.

4. TRIM AS REQUIRED

5. MINIMUM *4 BAR BOTH WAYS

6. TIE REBAR TOGETHER AT INTERSECTION POINTS

PANEL REINFORCING LAYOUT

OPTIONAL REBAR

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM HILFIKER SQUARE PANEL

	Names	Dates	Approve	d By /)	12/1	
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
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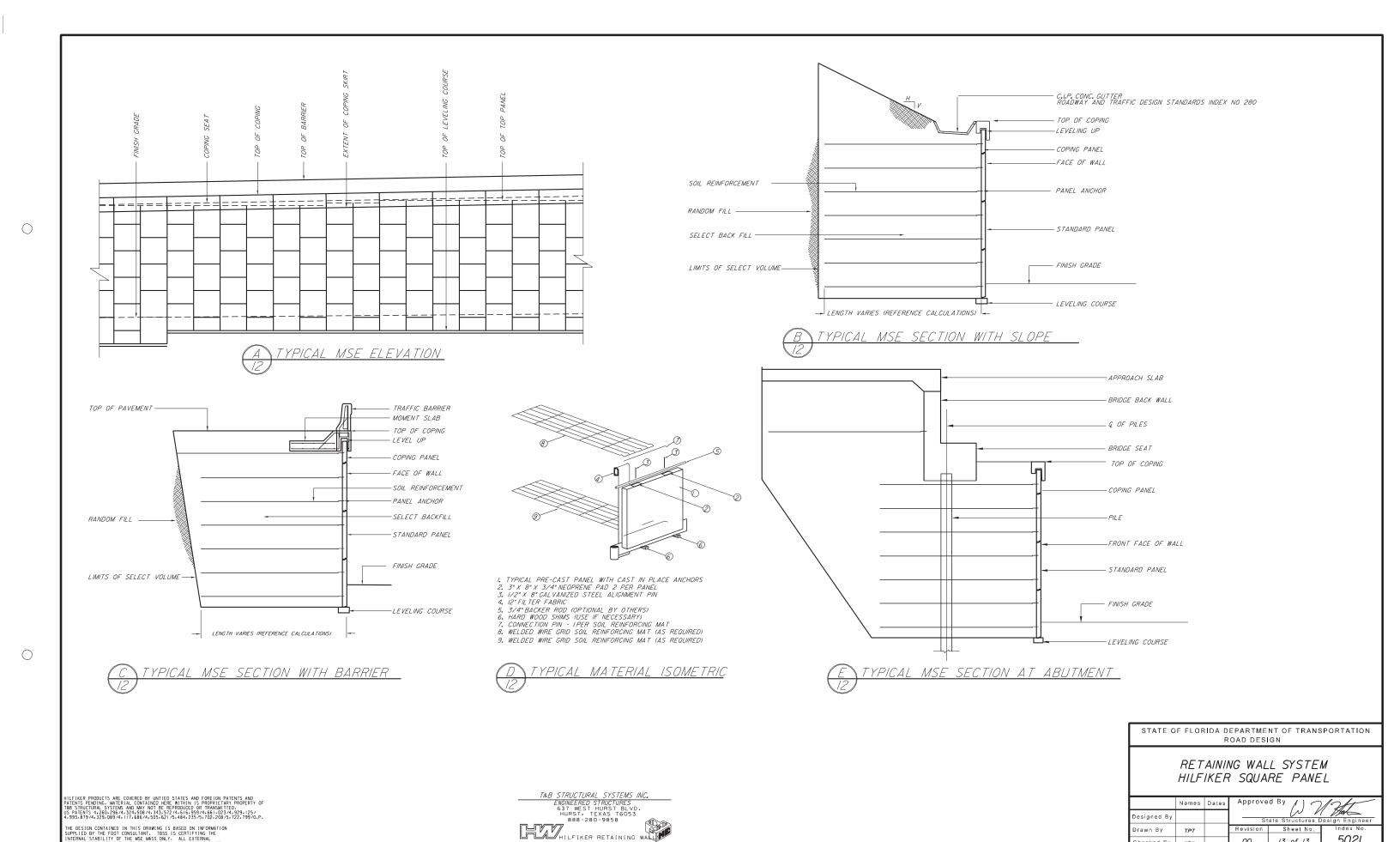
\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$ \$\$\$\$\$\$\$\$\$YT/MF\$\$\$\$\$\$

TYPE D - 3'-6" PANEL HILFIKER PRODUCTS ARE COVERED BY UNTIED STATES AND FOREIGN PATENTS AND PATENTS PENDING. MATERIAL CONTAINED HERE WITHIN 15 PROPRETATY PROPERTY OF TAB STRUCTURAL SYSTEMS AND MAY NOT BE REPRODUCED ON TRANSMITTED. U.S. PATENTS 4,260.296.44.374.508.454.343.572.44.616.3594.661.02374.929.125.4993.879.44.329.0894.4117.86874.505.62175.424.525.57.702.208.75.702.2087.2087.2087.019.

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\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$ \$\$\$\$\$\$\$SYT/ME\$\$\$\$\$

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CONSTRUCTION NOTES FOR PLACEMENT OF TENSAR GEOGRIDS AND BACKFILL SOILS FOR TENSAR PRECAST CONCRETE REINFORCED WALLS TENSAR MSE RETAINING WALL SYSTEM

1.0 MATERIALS

- I.I GEOGRID REINFORCING SHALL BE TENSAR BIAXIAL AND UNIAXIAL GEOGRIDS MANUFACTURED BY THE TENSAR CORPORATION, MORROW, GEORGIA.
- I.2 BODKIN BARS SHALL BE 1/2" x//4" HDPE BARS MANUFACTURED BY THE TENSAR CORPORATION, MORROW, GEORGIA.
- .3 DRAINAGE MATERIALS
- I.3.I GEOTEXTILE TG600 FABRIC SHALL BE MANUFACTURED BY EVERGREEN TECHNOLOGIES, INC., EVERGREEN, ALABAMA, OR EQUIVALENT AS APPROVED BY THE ENGINEER.
- 2.0 TECHNICAL REQUIREMENTS
- 2.1 FILL MATERIALS SHALL BE PLACED FROM THE BACK FACE OF THE WALL TOWARDS THE TAILS OF THE GEOGRID TO ENSURE FURTHER TENSIONING.
- 2.2 FILL SHALL BE COMPACTED AS SPECIFIED IN SECTION 548 OF THE PROJECT SPECIFICATIONS.
- 2.3 AN APPROVED SET OF CONSTRUCTION DRAWINGS AND CONTRACT SPECIFICATIONS SHALL BE ON-SITE AT ALL TIMES, DURING CONSTRUCTION OF THE TENSAR RETAINING WALL.
- 3.0 TENSAR GEOGRID PLACEMENT
- 3.1 TENSAR GEOGRID SHALL BE PLACED AT THE LOCATIONS AND ELEVATIONS SHOWN ON THE SHOP DRAWINGS.
- 3.2 TENSAR GEOGRID LENGTH SHALL BE AS SHOWN ON THE CONSTRUCTION DRAWINGS. REINFORCED FILL ZONE LENGTH IS MEASURED FROM THE BACK FACE OF THE CONCRETE PANEL, EXTENDING TO THE TAIL OF THE GEOGRIDS.
- 3.2.1 TENSAR GEOGRID REINFORCEMENT SHALL BE CONTINUOUS THROUGHOUT THEIR EMBEDMENT LENGTH(S). THE BODKIN CONNECTION SHALL NOT BE UTILIZED UNLESS PRE-APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION.
- 3.2.2 IF PRE-APPROVED, TENSAR UNIAXIAL GEOGRIDS MAY BE SPLICED UTILIZING THE BODKIN CONNECTION DETAIL. NO MORE THAN ONE SPLICE SHALL BE ALLOWED IN ANY ONE IFNGTH OF REINFORCING.
- 3.3 PRIOR TO PLACING FILL, THE GEOGRID MATERIALS SHALL BE CONNECTED TO THE PANELS PER PANEL CONNECTION DETAIL (SEE TYPICAL DETAILS) AND PULLED TAUT AND ANCHORED TO REMOVE ANY SLACK IN THE GEOGRIDS.

- 3.4 TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID. A MINIMUM FILL THICKNESS OF SIX INCHES IS REQUIRED FOR OPERATION OF TRACKED VEHICLES OVER THE GEOGRID. TURNING OF TRACKED VEHICLES SHOULD BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND/OR THE GEOGRID.
- 3.5 RUBBER-TIRED VEHICLES MAY PASS OVER THE GEOGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE AVOIDED.
- 3.6 TENSAR UNIAXIAL GEOGRID SHALL BE ROLLED OUT WITH THE LONG AXIS OF THE APERTURES (MACHINE DIRECTION) PERPENDICULAR TO THE WALL FACE. TENSAR BIAXIAL GEOGRIDS SHALL BE ROLLED OUT WITH THE MACHINE DIRECTION BAR PARALLEL TO THE WALL FACE.
- 4.0 CHANGES TO GEOGRID LAYOUT OR PLACEMENT
- 4.1 NO CHANGES TO THE TENSAR GEOGRID LAYOUT, INCLUDING, BUT NOT LIMITED TO, LENGTH, GEOGRID TYPE, OR ELEVATION, SHALL BE MADE WITHOUT THE EXPLICIT WRITTEN CONSENT OF TENSAR EARTH TECHNOLOGIES, INC.
- 5.0 DRAINAGE
- 5.1 AT THE END OF EACH WORK DAY, BACKFILL SURFACE SHALL BE GRADED AWAY FROM THE WALL FACE A MINIMUM OF 2 PERCENT SLOPE AND A TEMPORARY SOIL BERM SHALL BE CONSTRUCTED NEAR THE WALL CREST TO PREVENT SURFACE WATER RUNOFF FROM OVERTOPPING THE WALL.
- 5.2 AT THE END OF EACH WORK DAY, BACKFILL SURFACE SHALL BE COMPACTED WITH A SMOOTH WHEEL ROLLER TO MINIMIZE PONDING OF WATER AND SATURATION OF THE BACKFIII.
- 5.3 THE TENSAR WALL HAS BEEN DESIGNED ON THE ASSUMPTION THAT THE REINFORCED FILL MATERIAL SHALL BE FREE OF SUBSURFACE DRAINAGE OF WATER (SEEPAGE).
- 5.4 THE CONTRACTOR SHALL BE RESPONSIBLE FOR WATER RETENTION AS NEEDED DURING CONSTRUCTION.
- 5.0 DESIGN PARAMETERS
- 6.1 SOIL PARAMETERS

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF FRICTION ANGLE, APPARENT COHESION AND UNIT WEIGHT SHALL BE PROVIDED IN THE SHOP DRAWINGS.

6.1.1 DESIGN:

THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY OTHERS. ON THE BASIS OF THIS INFORMATION, THE TENSAR CORPORATION IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

- 6.2 FACTORS OF SAFETY:
- 6.2.I INTERNAL STABILITY:

 MAXIMUM GEOGRID DESIGN STRENGTH = 0.19 ULT

 MINIMUM FACTOR OF SAFETY FOR GEOGRID PULLOUT = 1.5

 LOWEST GEOGRID

 SOIL-GEOGRID INTERACTION COEFFICIENT = 0.8

 PERCENT COVERAGE OF GEOGRID:

 (ONE ROLL WIDTHS) = 89%,

 (ONE-HALF ROLL WIDTH) = 44%
- 6.2.2 EXTERNAL STABILITY:

MINIMUM FACTOR OF SAFETY FOR SLIDING AT BASE = 1.
MINIMUM FACTOR OF SAFETY FOR OVERTURNING = 2.0
MINIMUM FACTOR OF SAFETY FOR BEARING = 2.5

(EXTERNAL STABILITY, INCLUDING SLIDING, OVERTURNING, AND BEARING CAPACITY, IS THE RESPONSIBILITY OF OTHERS. TENSAR EARTH TECHNOLOGIES, INC. ACCEPTS NO LIABILITY OR RESPONSIBILITY FOR EXTERNAL STABILITY. (SEE NOTES 7.6 & 7.7))

6.2.3 GLOBAL STABILITY:

MINIMUM FACTOR OF SAFETY FOR GLOBAL STABILITY = 1.5

= NONE

GLOBAL STABILITY IS THE RESPONSIBILITY OF OTHERS. TENSAR EARTH TECHNOLOGIES, INC. ACCEPTS NO LIABILITY OR RESPONSIBILITY FOR GLOBAL STABILITY. (SEE NOTES 7.6 & 7.7)

6.3 SURCHARGE LOADING = 250 psf

6.4 HYDROSTATIC DESIGN = NONE

6.6 GEOGRID LONG TERM ALLOWABLE DESIGN STRENGTH (LTADS): GEOGRID LTADS SHALL BE 19 PERCENT OF ULTIMATE GEOGRID STRENGTH AS DETERMINED IN ACCORDANCE WITH GEOSYNTHETIC RESEARCH INSTITUTE, (GRI), TEST METHOD

GGI-87, SINGLE RIB TEST.

7.0 SPECIAL PROVISIONS

6.5

SEISMIC DESIGN

- 7.1 WALL ELEVATION VIEWS AND LOCATIONS AND GEOMETRY OF EXISTING STRUCTURES MUST BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.
- 7.2 TENSAR EARTH TECHNOLOGIES, INC. ASSUMES NO LIABILITY FOR INTERPRETATION OR VERIFICATION OF SUBSURFACE CONDITIONS, SUITABILITY OF SOIL DESIGN PARAMETERS AND INTERPRETATION OF SUBSURFACE GROUNDWATER CONDITIONS.
- 7.3 THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING AND VERIFYING THAT THE ACTUAL SITE CONDITIONS ARE AS DESCRIBED IN SECTION 6.0 PRIOR TO AND DURING CONSTRUCTION. THE ENGINEER SHALL BE ON-SITE TO ASSURE THE PROVISIONS IN THE CONSTRUCTION NOTES ARE FOLLOWED.

7.4 THE SOIL DESIGN PARAMETERS STATED IN SECTION 6.0 SHALL BE VERIFIED BY THE CONSTRACTOR. WRITTEN VERIFICATION OF DESIGN PARAMETERS SHALL BE SUBMITTED TO TENSAR EARTH TECHNOLOGIES, INC. PRIOR TO COMMENCING WITH CONSTRUCTION.

7.5 ANY REVISIONS TO DESIGN PARAMETERS STATED IN SECTION 6.0 OR STRUCTURE GEOMETRY SHALL REQUIRE DESIGN MODIFICATIONS PRIOR TO PROCEEDING WITH CONSTRUCTION

7.6 PER THE MSE RETAINING WALL GENERAL NOTES, TENSAR EARTH TECHNOLOGIES, INC HAS CONSIDER INTERNAL STABILITY OF THE RETAINING WALLS ONLY EXTERNAL AND GLOBAL STABILITY OF THE WALL IS THE RESPONSIBILITY OF OTHERS

7.7 DIFFERENTIAL SETTLEMENT AND ITS EFFECTS ON THE TENSAR RETAINING WALL SYSTEM SHALL BE THE RESPONSIBILITY OF OTHERS.

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS AS NOTED IN THESE PLANS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

	Names	Dates	Approve	d By	12/	ı
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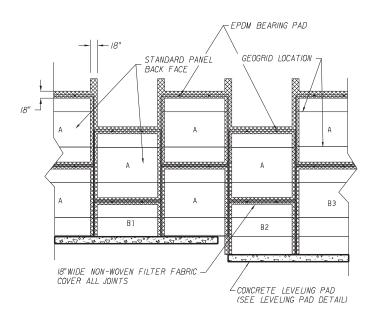
THIS DESIGN IS BASED UPON SPECIFIC PROPERTIES OF TENSAR PRODUCTS (GEOGRIDS, DRAINAGE COMPOSITES AND EROSION MEDIA), WHICH ARE PROPRIETARY TO THE TENSAR CORPORATION 1210 CITIZENS PARKWAY, MORROW GA. 30260. ANY SUBSTITUTION OF THE SPECIFIED PRODUCTS WILL INVALIDATE THIS DESIGN.

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TYPICAL FILTER FABRIC COVERAGE DETAIL

NOT TO SCALE

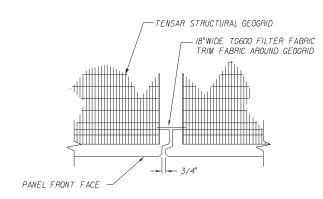
PANEL FRONT FACE

TENSAR STRUCTURAL GEOGRID

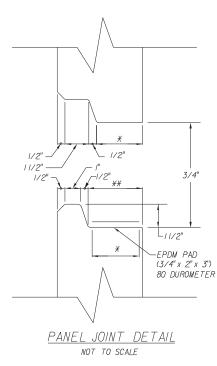
18"WIDE EVERGREEN TG600 GEOTEXTILE

EPDM PAD
(2 PER PANEL, (TYP.)

HORIZONTAL JOINT DETAIL
NOT TO SCALE



VERTICAL JOINT DETAIL



* - 3"FOR MODERATELY & SLIGHTLY

AGGRESSIVE ENVIRONMENT

- 4 1/2" FOR EXTREMELY AGGRESSIVE ENVIRONMENT

** - 3 1/2" FOR MODERATELY & SLIGHTLY
AGGRESSIVE ENVIRONMENT

- 4 3/4" FOR EXTREMELY AGGRESSIVE ENVIRONMENT

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM
TENSAR EARTH TECHNOLOGIES
MSE RETAINING WALL

	Names	Dates	Approved By ()		
Designed By			State Structures Design Engine		
Drawn By	JMS	8/14/98	Revision	Sheet No.	Index No.
Checked By			00	2 of 17	5025

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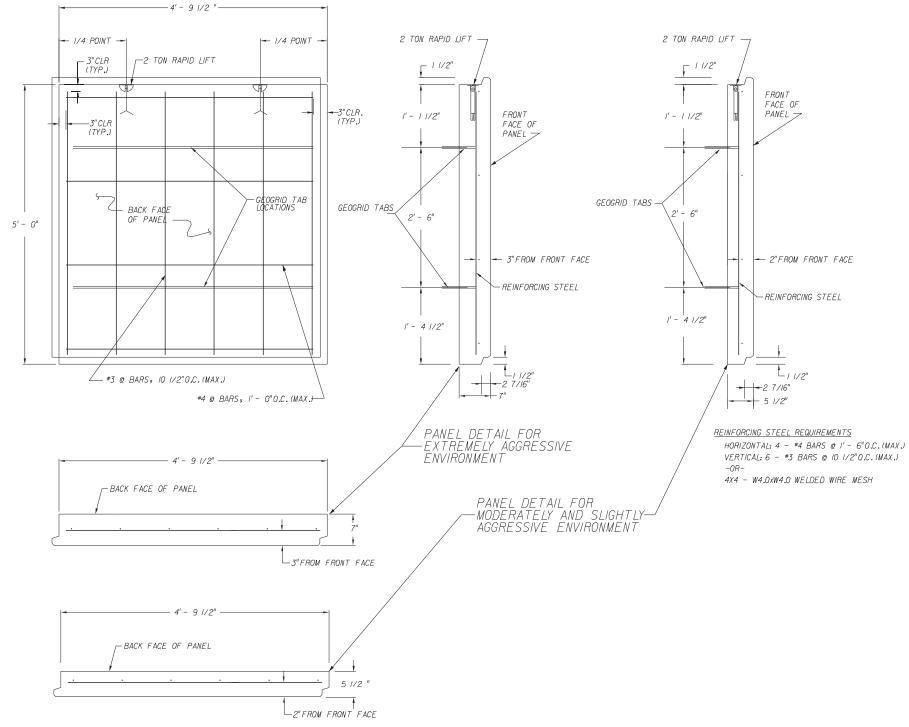
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TENSAR EARTH TECHNOLOGIES INC. 5775-B Glenridge Dirve Lakeside Center Suite 450 Atlanta, GA 30328 (404) 250-1290





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TYPICAL PANEL DETAILS - STANDARD A PANEL

TENSAR
EARTH TECHNOLOGIES. INC.
5775-B Glenridge Drive
Lakeslde Center Sulte 450
Atlanta, GA 30328
(404) 250-1290

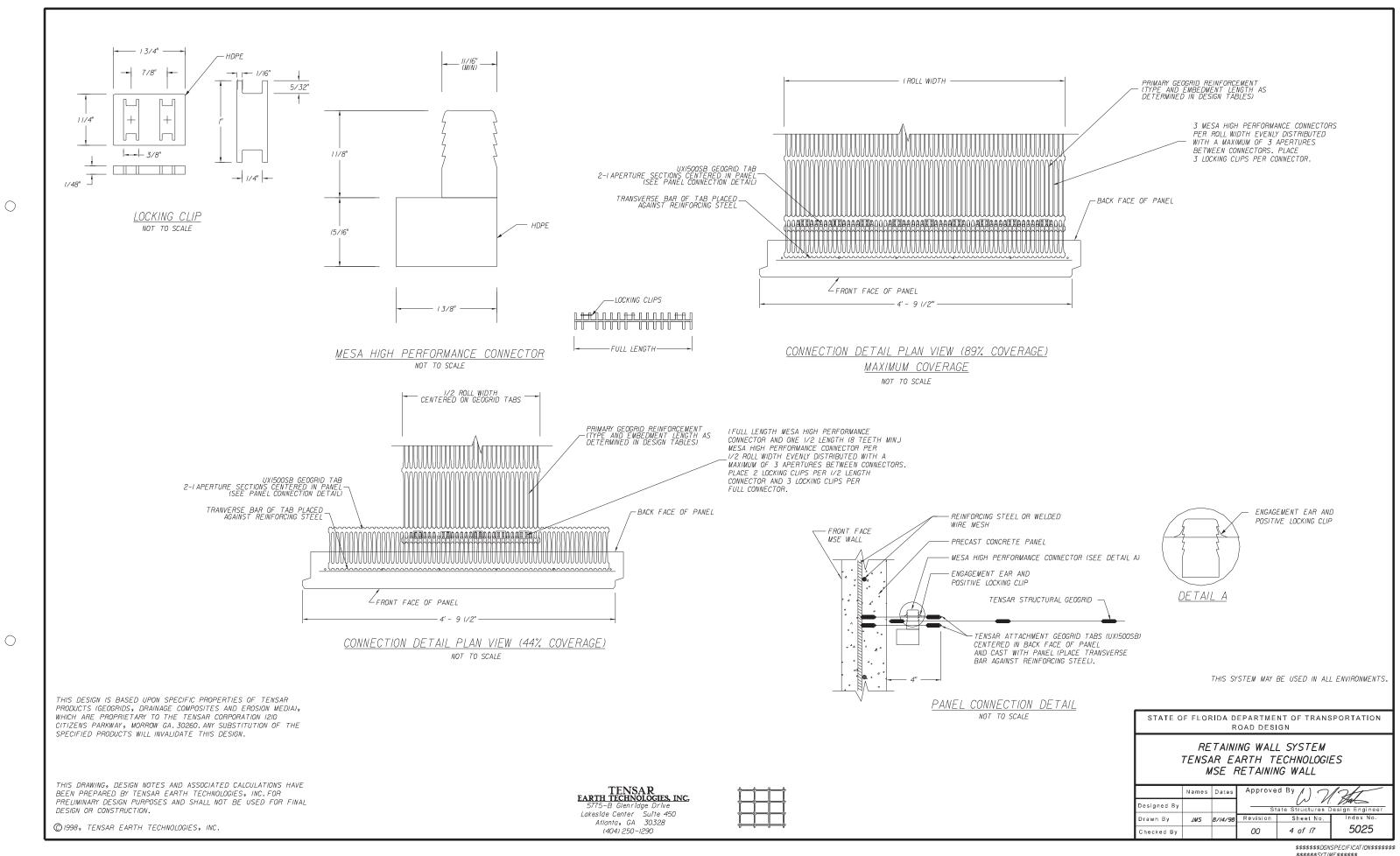


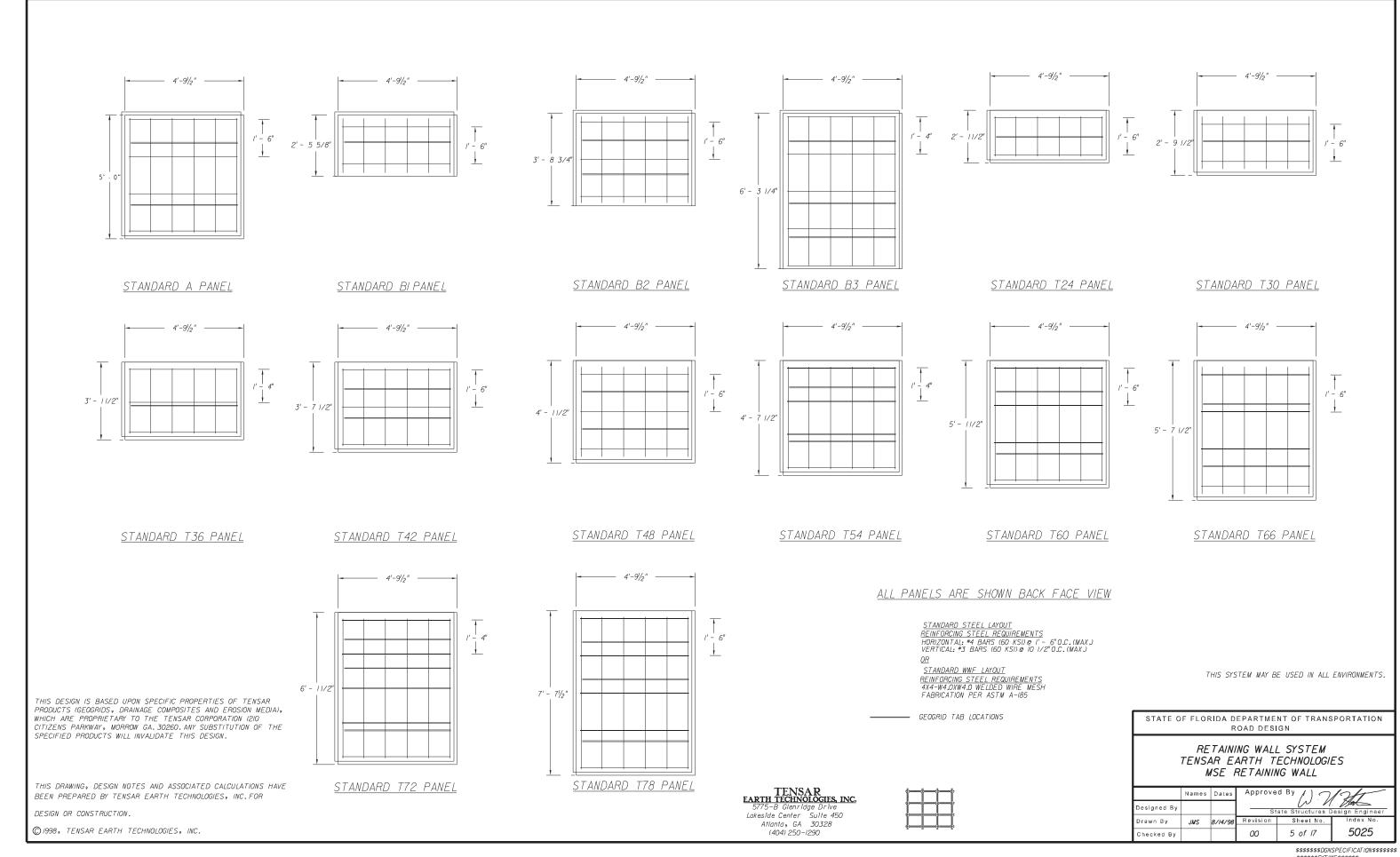
THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

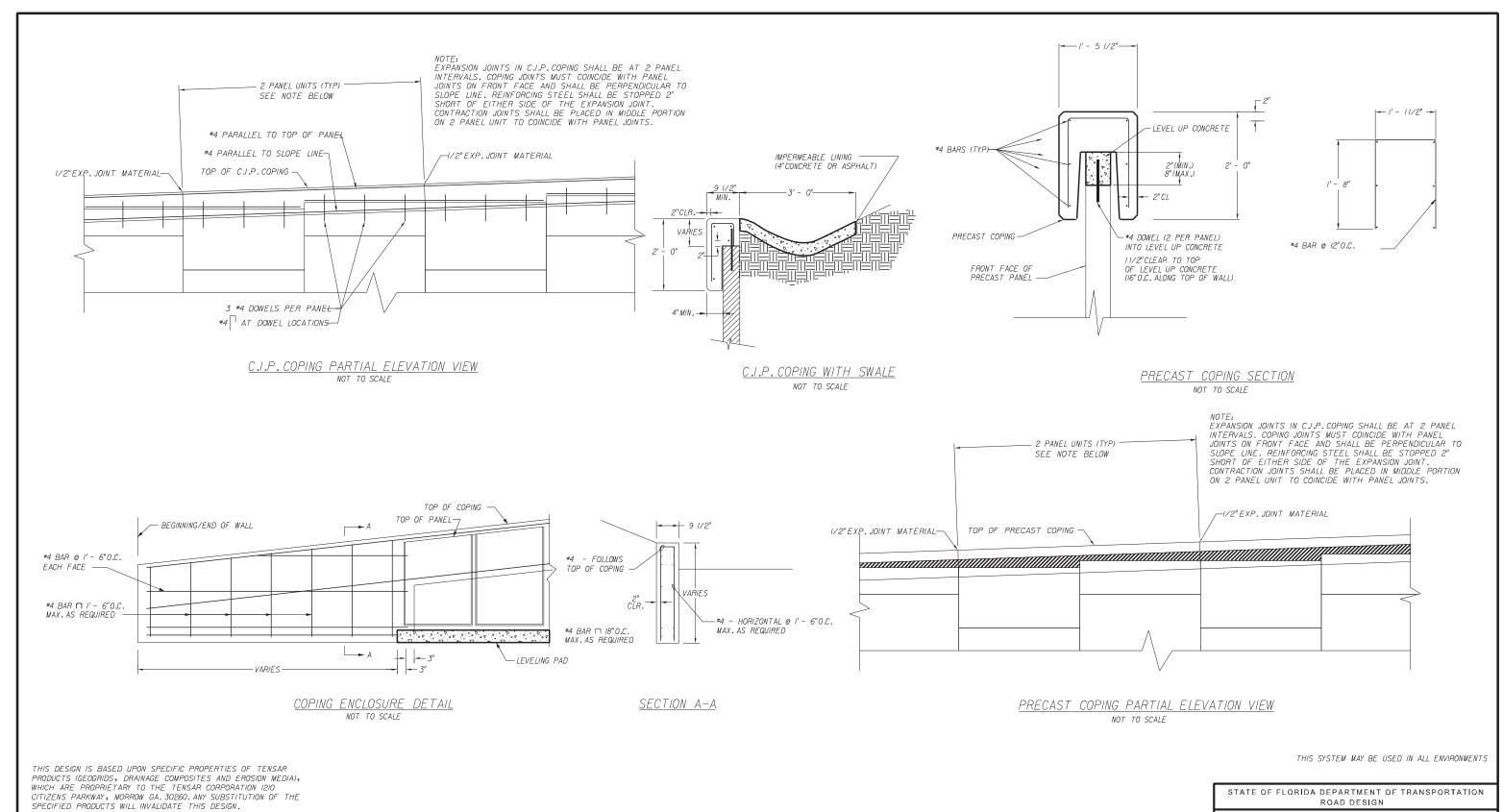
RETAINING WALL SYSTEM
TENSAR EARTH TECHNOLOGIES
MSE RETAINING WALL

	Names	Dates	Approved By		
Designed By			St	ate Structures D	esign Engineer
Drawn By	JMS	8/14/98	Revision	Sheet No.	Index No.
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STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

> RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

Names Dates Approved By Designed By rawn By JM5 8/14/98 5025 00 hecked By

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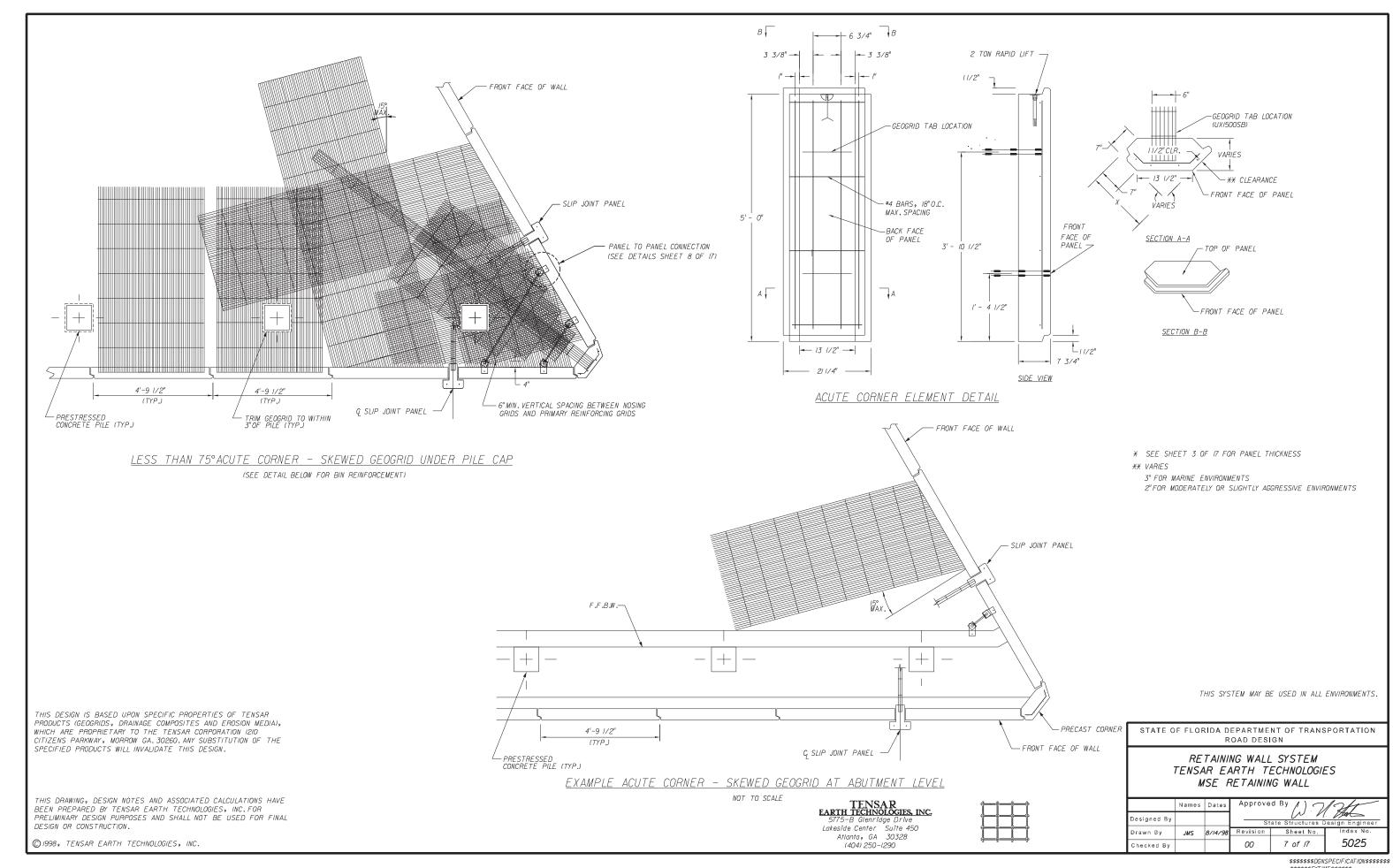
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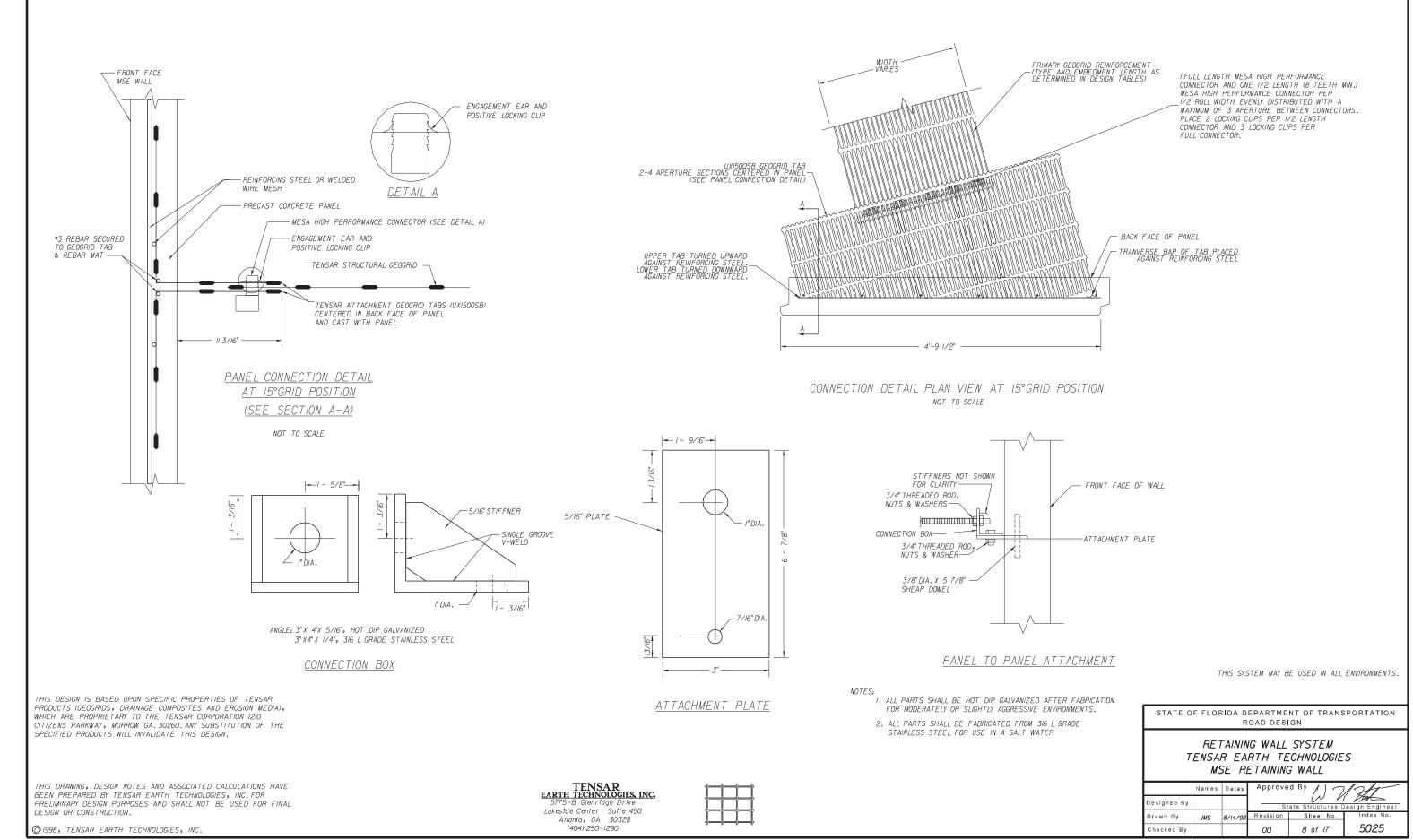
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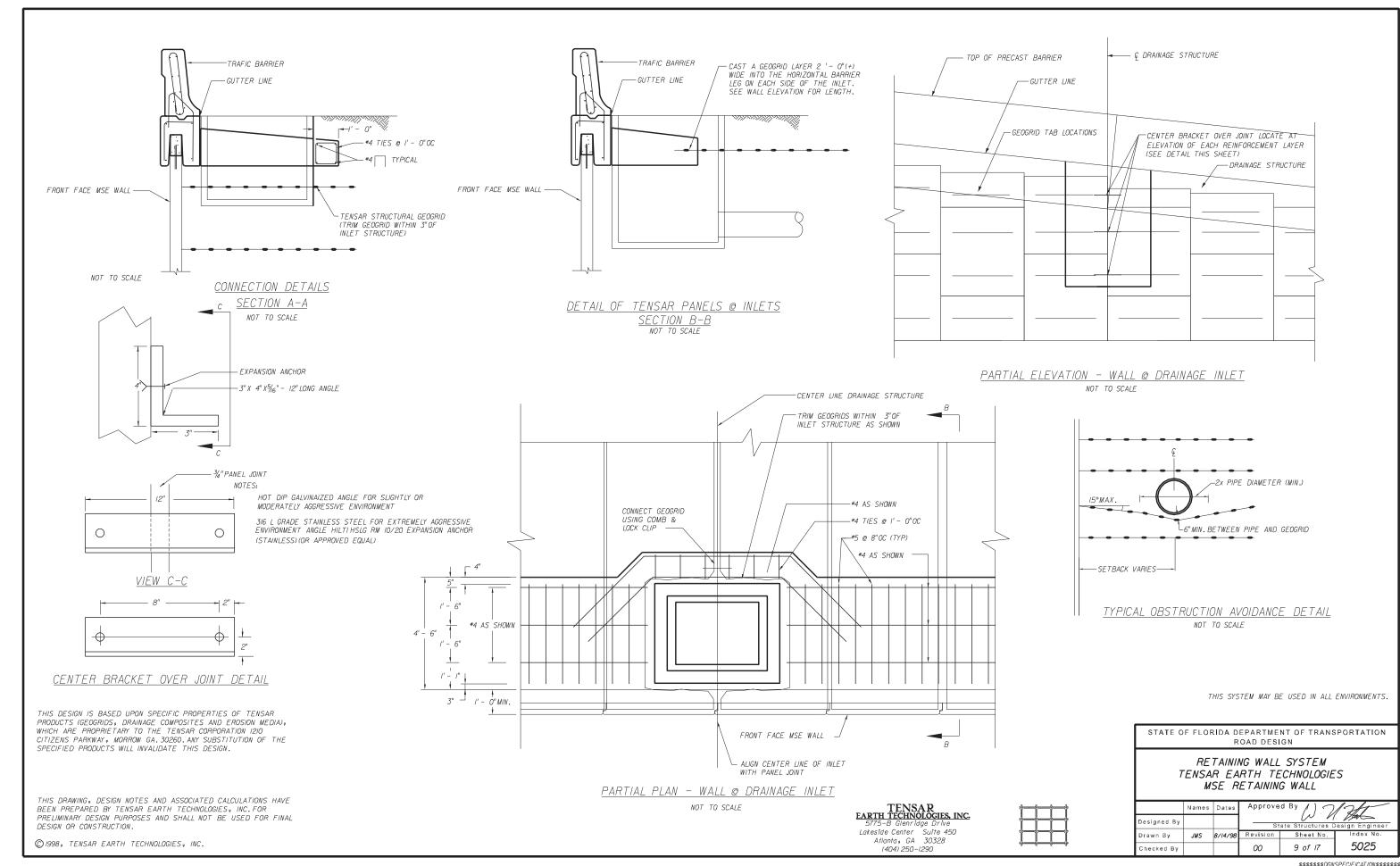
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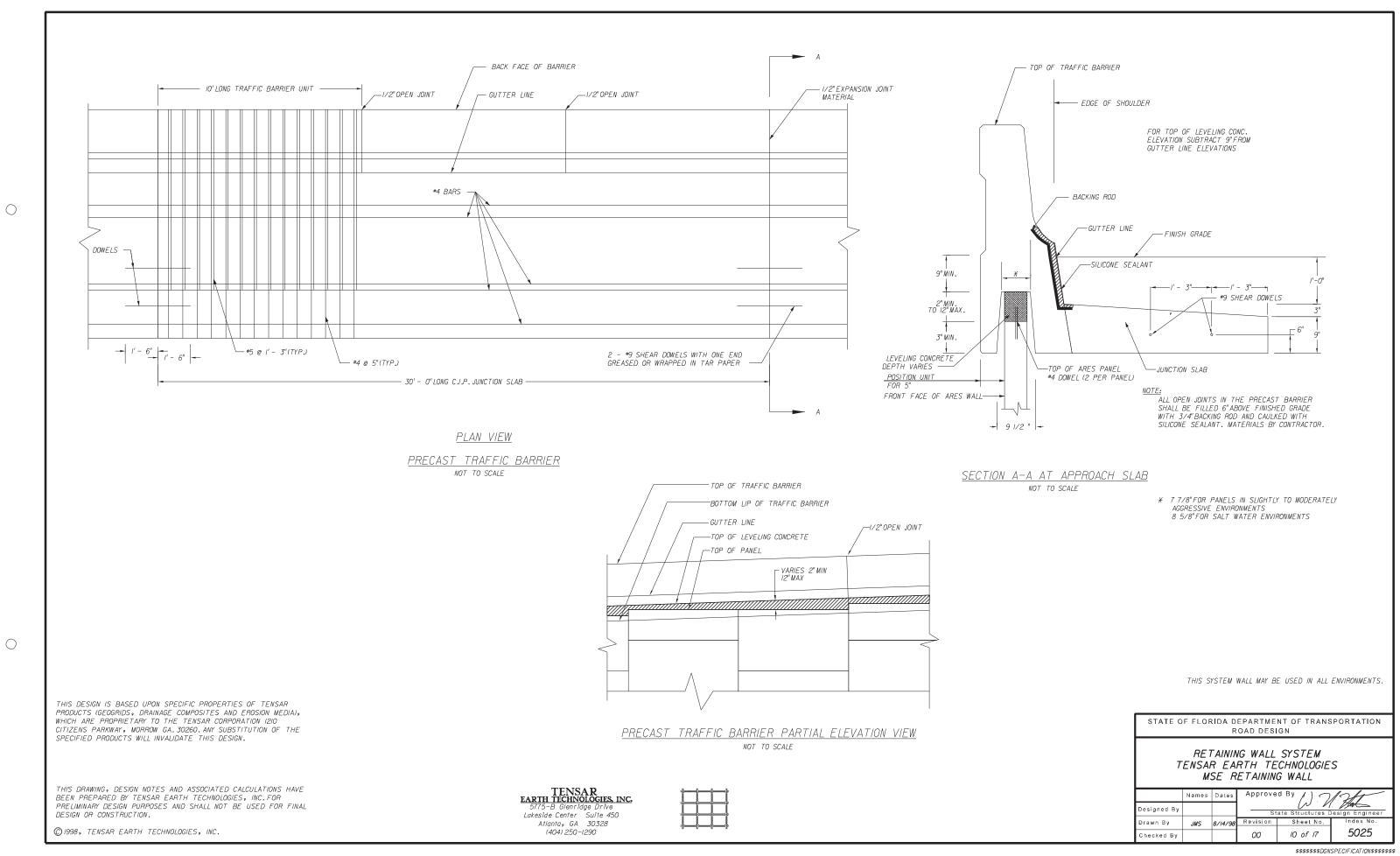
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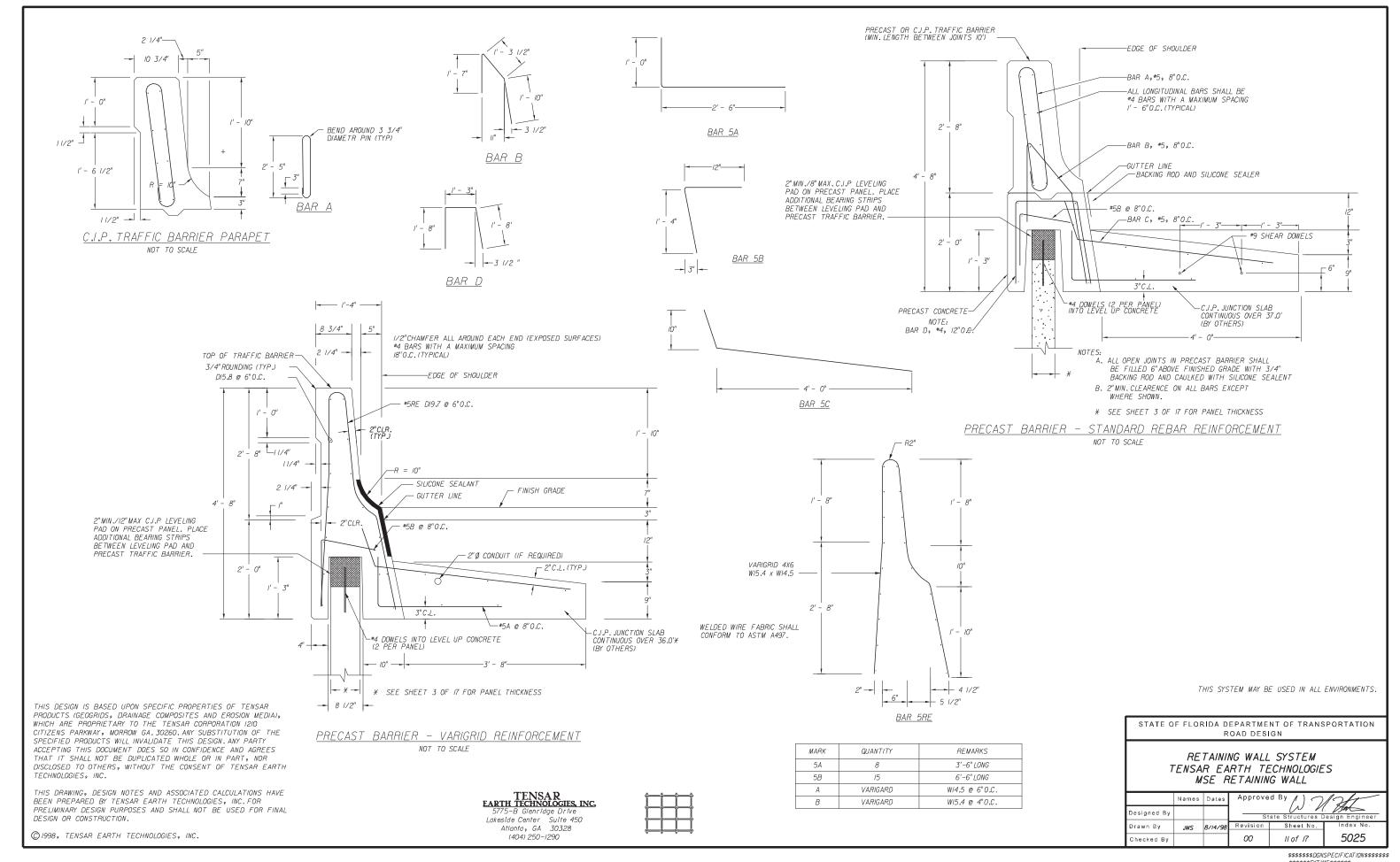


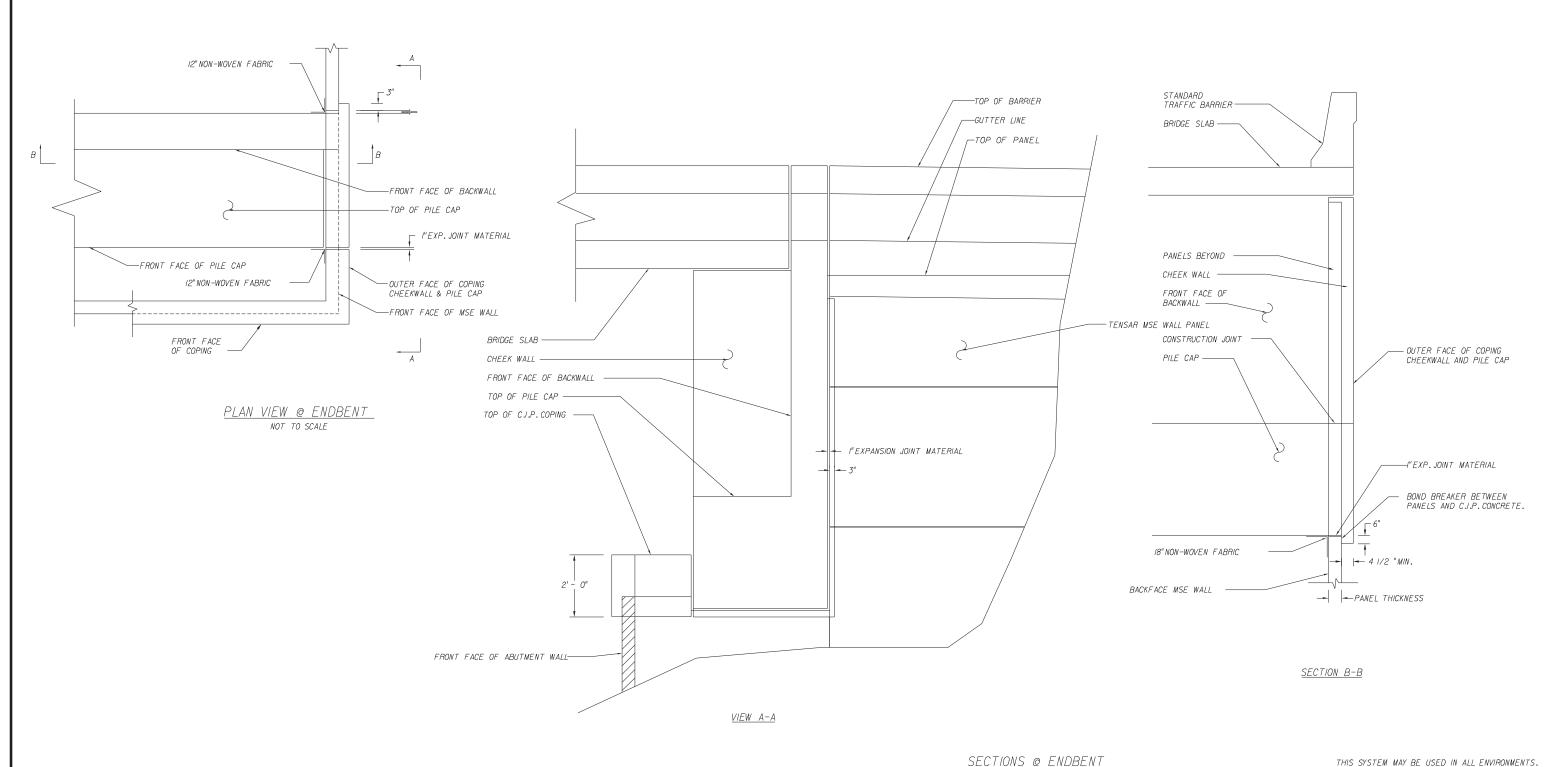












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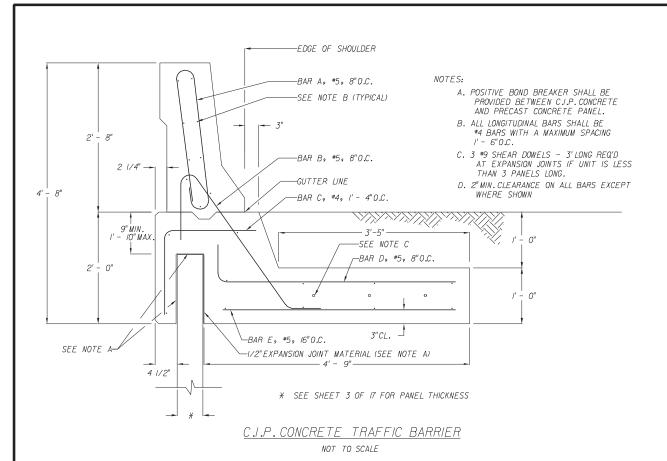
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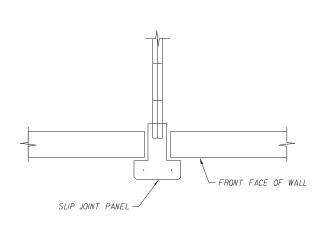
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

> RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

	Names	Dates	Approve	d By ()	12/
Designed By			St	tate Structures [Design Engineer
Drawn By	JMS	8/14/98	Revision	Sheet No.	Index No.
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- #4 BARS

SLIP JOINT DETAIL NOT TO SCALE

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GEOGRID LOCATION -STANDARD PANEL FRONT FACE EPDM BEARING PAD В3 ВЗ В2 SINGLE STEP I' - 3 3/16 "-9"(TYP.) - DOUBLE STEP 2' - 6 3/8

TOP OF TRAFFIC BARRIER -__ 1/2 "JOINT - 1/2 "OPEN JOINT "EXPANSION JOINT MATERIAL -GUTTER LINE TOP OF PANEL 2' - 0" (MIN.) _2" M/N. SLIP JOINT PANEL -

F OF SLIP JOINT PANEL-

C.I.P. TRAFFIC BARRIER OVER SLIP JOINT PANEL

CONCRETE LEVELING PAD (SEE LEVELING PAD DETAIL) TYPICAL LEVELING PAD STEP DETAIL NOT TO SCALE

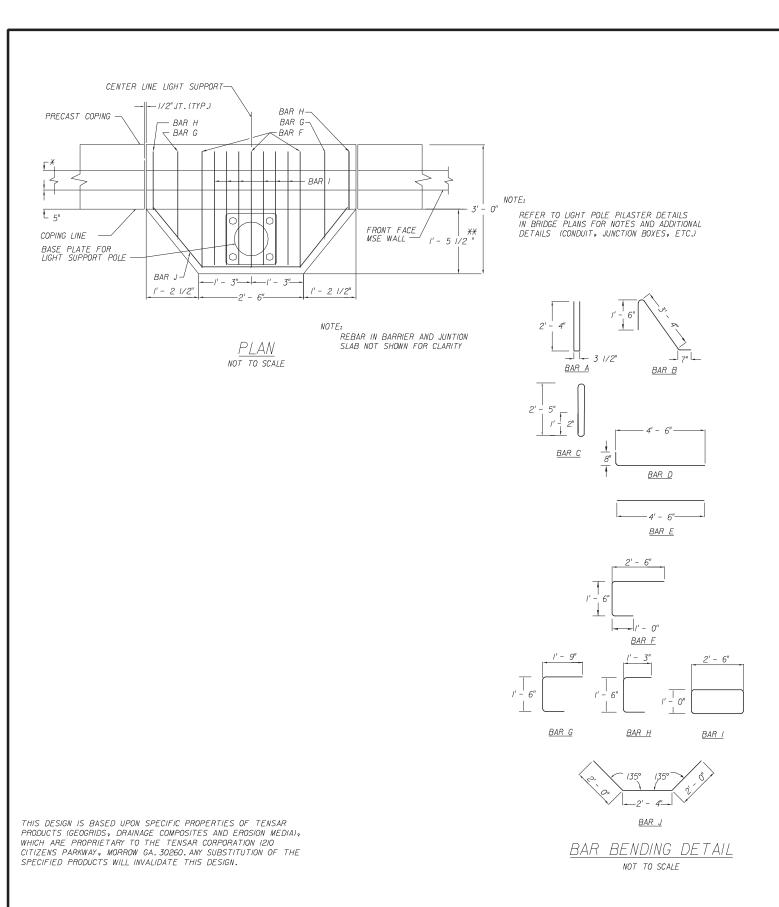
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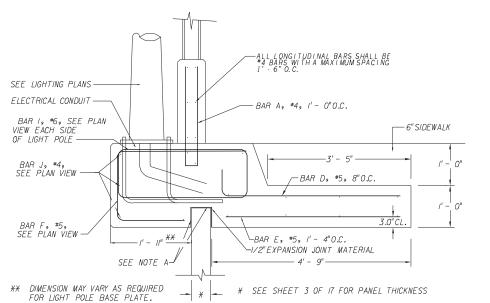
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

> RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES
> MSE RETAINING WALL

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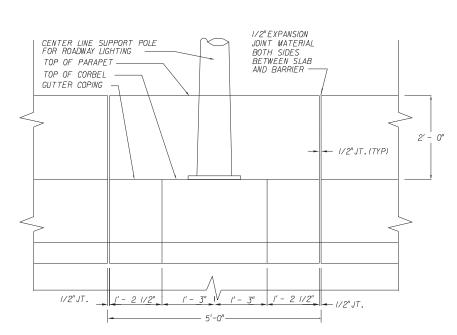




NOTES:
A. POSITIVE BOND BREAKER (6 MIL.POLYETHYLENE OR APPROVED EQUALISHALL BE PROVIDED BETWEEN CAST IN PLACE CONC. AND PRECAST CONC. PANEL.

B. MAINTAIN A 2" MIN. CLEARENCE ON ALL BARS. EXCEPT WHERE NOTED OTHERWISE.

PARAPET DETAIL AT LIGHT POLE NOT TO SCALE



PARTIAL ELEVATION

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

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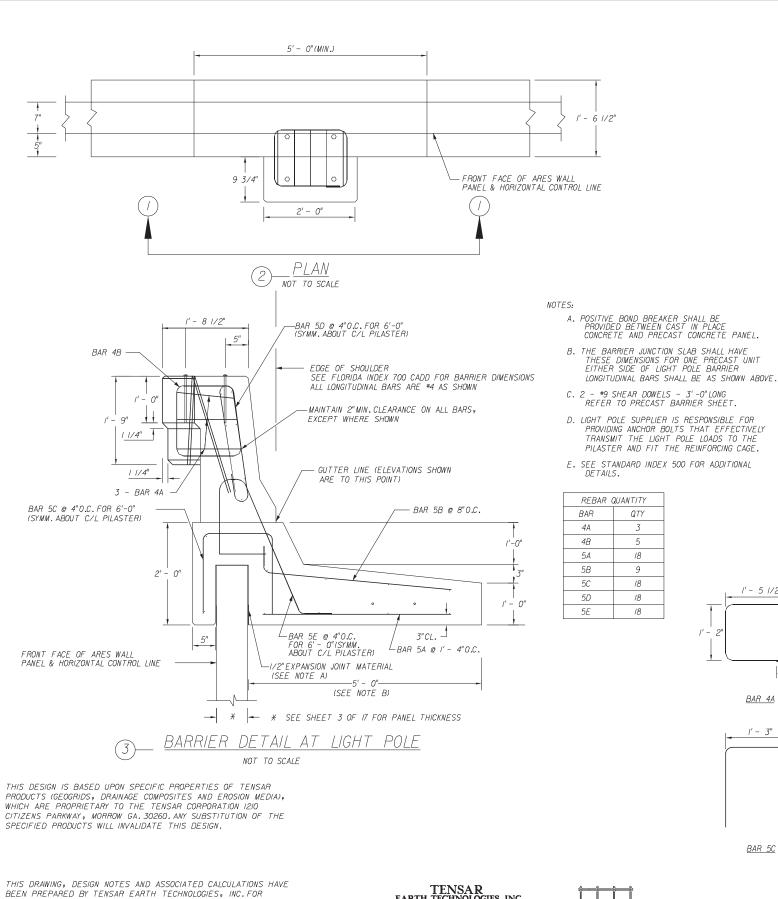


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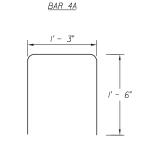
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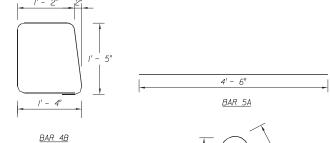
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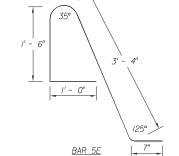
TOP OF BARRIER -PARTIAL ELEVATION

1' - 5 1/2" 4 1/2"



<u>BAR 5C</u>





SUPPORT POLE FOR ROADWAY LIGHTING

1/2" JOINT - OPEN WITH BACKING ROD AND SILICONE SEALANT - REFER TO SHEET WITH PRECAST BARRIER.

1/2" JOINT (TYP.)

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

4' - 6"

<u>BAR 5B</u>

RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

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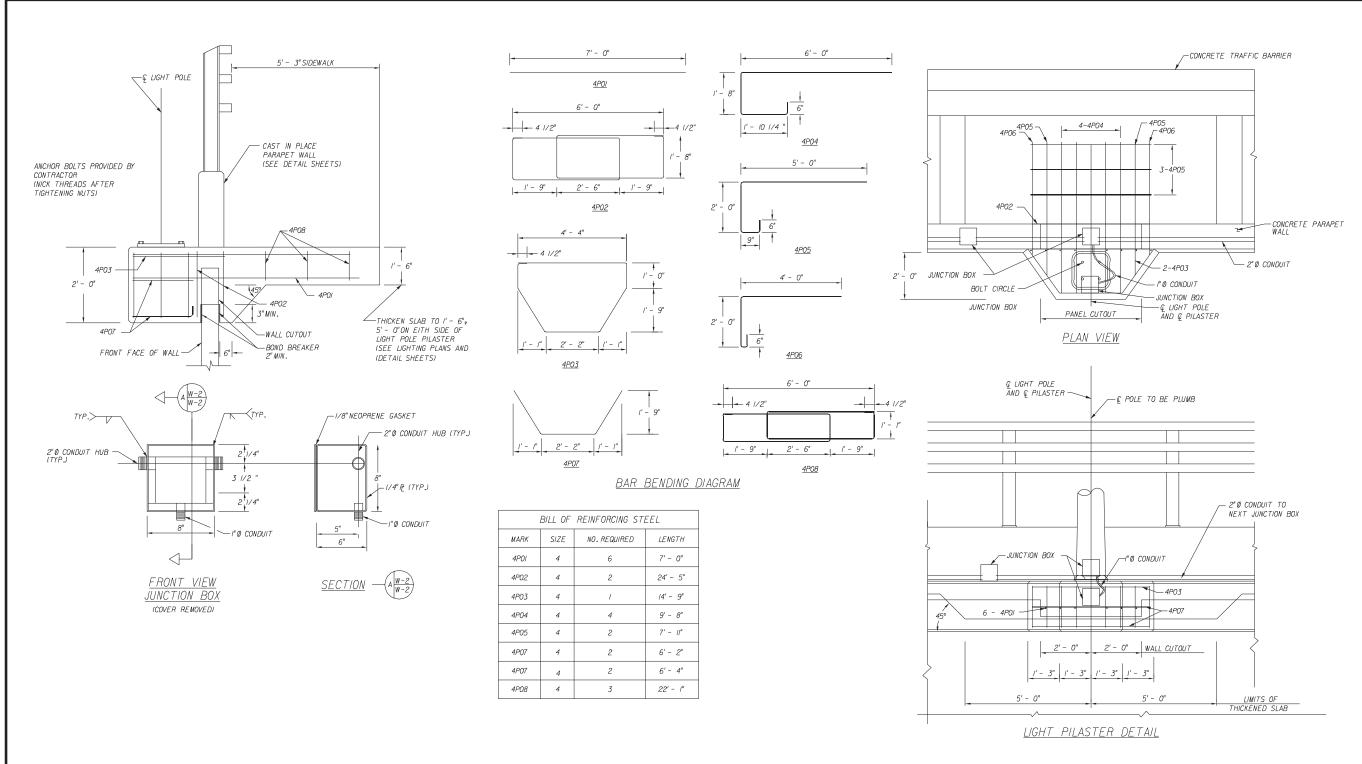
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BAR BENDING DETAIL NOT TO SCALE

BEND AROUND 3 3/4"DIA.PIN (TYP.)

<u>BAR 5D</u>

/' - 2"



THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM
TENSAR EARTH TECHNOLOGIES
MSE RETAINING WALL

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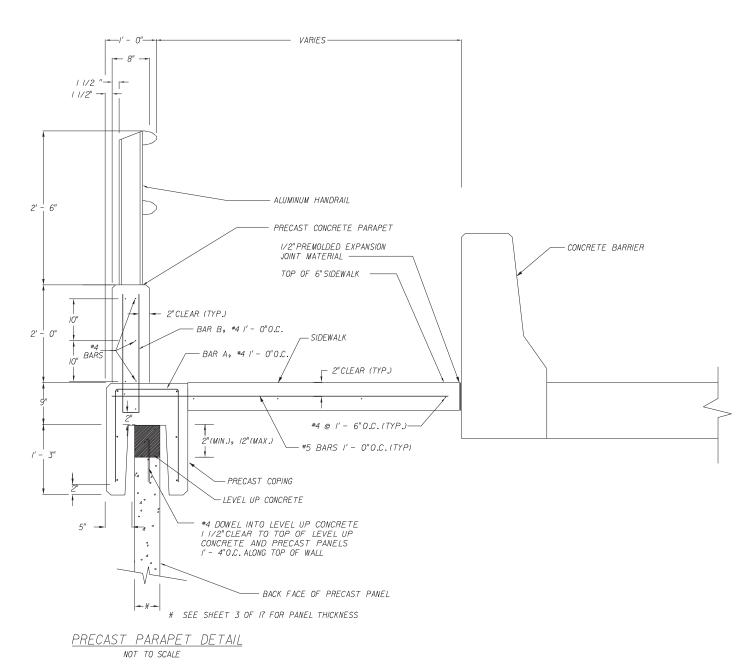
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1 1/2" 1 1/2" -ALUMINUM HANDRAIL -CAST IN PLACE PARAPET 1/2" PREMOLDED EXPANSION - CONCRETE BARRIER JOINT MATERIAL -TOP OF 6" SIDEWALK - 2"CLEAR (TYP) #4 #5 @ 1' O.C. \[2"CLEAR (TYP.) #4 @ 1' - 6"O.C.(TYP) - #4 DOWEL (2 PER PANEL) PROVIDE A POSITIVE BOND BREAKER BETWEEN C.J.P. CONCRETE AND PRECAST PANELS -BACK FACE OF PRECAST PANEL * SEE SHEET 3 OF 17 FOR PANEL THICKNESS

C.I.P. PARAPET DETAIL

NOT TO SCALE

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES MSE RETAINING WALL

 Designed By
 Dates
 Approved By
 State Structures Design Engineer

 Drawn By
 JMS
 8/14/98
 Revision
 Sheet No.
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 Checked By
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GENERAL NOTES

DESIGN CRITERIA

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I. DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL WITHIN THE REINFORCED EARTH VOLUME, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED MATERIALS SHALL CONFORM TO THE CONTRACTING AGENCY'S TECHNICAL SPECIFICATIONS FOR RETAINED EARTH WALLS.

2. FACTORS OF SAFETY

OVERTURNING 2.0 INTERNAL PULLOUT 1.5 (ALLOW DEFORMATION 3/4") OVERALL STABILITY 1.5 SLIDING 1.5 BEARING 2.5

SOIL REINFORCEMENT MESH 0.47 FV AT END OF DESIGN LIFE

3. SOIL CHARACTERISTICS ASSUMED FOR DESIGN:

SOIL PARAMETERS:

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF 0, C AND & SHALL BE PROVIDED IN THE SHOP DRAWINGS.

- 4. THE MAXIMUM APPLIED BEARING PRESSURE AT THE FOUNDATION LEVEL IS AS SHOWN ON THE WALL ELEVATIONS FOR EACH DESIGN CASE. IT IS THE RESPONSIBILITY OF OTHERS TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 5. ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED EARTH VOLUME, AS DETERMINED BY THE ENGINEER, SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL OR OTHERWISE STABILIZED AS DIRECTED BY THE ENGINEER.

WIRE FACING PANELS & REINFORCING ELEMENTS

6. REINFORCING MESH ELEMENTS SHALL BE SHOP FABRICATED FROM COLD DRAWN STEEL ROD CONFORMING TO THE MINIMUM REQUIREMENTS OF ASTM A-82 AND SHALL BE WELDED AT THE JUNCTIONS BETWEEN LONGITUDINAL AND TRANSVERSE WIRES IN ACCORDANCE WITH ASTM A-185. GALVANIZATION SHALL BE APPLIED AFTER MESH FABRICATION AND SHALL CONFORM TO THE MINIMUM REQUIREMENTS OF ASTM A-123.

LOOP EMBEDS SHALL BE FABRICATED FROM COLD DRAWN STEEL ROD COMFORMING TO ASTM A-510 OR ASTM A-82. LOOP EMBEDS SHALL BE WELDED IN ACCORDANCE WITH ASTM A-185. LOOP EMBEDS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM B-633.



A Division of L. B. Foster Company

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DESIGN:

7. THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY OTHERS. ON THE BASIS OF THIS INFORMATION. THE WALL COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

WALL CONSTRUCTION

- 8. RETAINED EARTH WALLS IN CURVES WILL FORM A SERIES OF SHORT CHORDS OF 10.0' EACH TO MATCH DESIRED WALL ALIGNMENT.
- 9. FOR LOCATION AND ALIGNMENT OF RETAINED EARTH WALLS. SEE RETAINING WALL CONTROL PLANS.
- 10. IF MANHOLES AND DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN ON WALL ELEVATIONS.
- II.IF PILES ARE LOCATED WITHIN REINFORCED SOIL VOLUME. THEY
 SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE REINFORCED EARTH WALL UNLESS A METHOD TO PROTECT THE STRUCTURE WHICH IS ACCEPTABLE TO THE ENGINEER AND FOSTER GEOTECHNICAL COMPANY AND IS PROPOSED AND APPROVED IN WRITING.
- 12. BACKELLI MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH SECTION 548 TO A LEVEL OF 2"(+/-) ABOVE THE TIE MESH EMBEDDED IN THE PANELS. INSTALLATION OF REINFORCING MESH SHALL BE PERMITTED ONLY AFTER PLACEMENT AND COMPACTION OF THE BACKFILL MATERIAL HAS REACHED THE REQUIRED LEVEL.
- 13. WALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH SECTION 548.
- 14.1T IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE LOCATION OF ANY GUARDRAIL POSTS BEHIND RETAINED EARTH PANELS.PRIOR
 TO PLACEMENT OF THE TOP LAYER OF REINFORCING MESH. INDIVIDUAL REINFORCING MESH MAY BE SKEWED TO AVOID THE POST LOCATIONS IF AUTHORIZED BY THE ENGINEER (NO CUTTING OF SOIL REINFORCEMENT GRIDS ALLOWED UNLESS SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER), ANY DAMAGE DONE TO THE REINFORCING MESH DUE TO THE INSTALLATION OF THE GUARDRAIL SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- 15. IF EXISTING OR FUTURE STRUCTURES, PIPES, FOUNDATIONS OR GUARDRAIL POSTS WHICH ARE WITHIN REINFORCED SOIL VOLUME INTERFERE WITH THE NORMAL PLACEMENT OF REINFORCING MESH AND SPECIFIC DIRECTION HAS NOT BEEN PROVIDED ON THE PLANS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO DETERMINE WHAT COURSE OF ACTION SHOULD BE TAKEN.
- 16. FOR OTHER INFORMATION PERTAINING TO WALL CONSTRUCTION PLEASE REFER TO FOSTER GEOTECHNICAL CONSTRUCTION MANUAL.
- 17. THE CONTRACTOR IS RESPONSIBLE FOR GRADUALLY DEFLECTING UPPER REINFORCING MESH DOWNWARD TO AVOID CONFLICTS WITH PAVING AND SUBGRADE PREPARATION. THE CONTRACTOR'S ATTENTION IS DIRECTED ESPECIALLY TO SITUATIONS WHERE ROADWAY SUPER ELEVATION AND/OR SOIL MIXING ARE ANTICIPATED.

MATERIALS NOTES

18. NOMINAL MESH LENGTHS

THE REINFORCING MESH LENGTH SHOWN ON THE PLANS, MEASURED FROM BACK FACE OF PANEL ARE THE NOMINAL LENGTHS REQUIRED BY CALCULATION. THE ACTUAL FABRICATED MESH LENGTHS ARE OFTEN LONGER (UP TO 6") DUE TO MANUFACTURING TOLERANCES. THE REQUIRED HORIZONTAL LIMIT OF GRANULAR BACKFILL IS EQUAL TO THE NOMINAL MESH LENGTH. ADDITIONAL GRANULAR BACKFILL BEYOND THE NOMINAL MESH LENGTH IS NOT REQUIRED BY CALCULATION.

19. SELECT BACKFILL QUANTITY

THE SELECT BACKFILL QUANTITY INDICATED BY FOSTER GEOTECHNICAL IS CALCULATED BY MULTIPLYING THE NOMINAL MESH LENGTHS SHOWN ON THE PLANS BY THEIR TRIBUTARY WALL SURFACE AREA AND CONVERTING THE RESULT TO A NEATER CUBIC METER QUANTITY. THIS INFORMATION IS FURNISHED FOR THE CONTRACTOR'S INFORMATION ONLY AND IS NOT INTENDED TO PRESENT THE ACTUAL QUANTITIES REQUIRED TO COMPLETE THE WORK. THE CONTRACTOR MUST CALCULATE HIS OWN EXCAVATION AND BACKFILL QUANTITIES BASED UPON THE SPECIFIC CONDITIONS OF THE PROJECT.

20. NOTE TO CONTRACTORS

ONLY THE FOLLOWING MATERIALS ARE SUPPLIED BY FOSTER GEOTECHNICAL

- PREFABRICATED FACING PANELS
- REINFORCING MESH
- NON-WOVEN FILTER CLOTH (FOR BEHIND FACING PANELS ONLY) (WEBTECH-TERRATEX NO4 OR EQUAL)

ANY OTHER MATERIALS CALLED FOR IN THE CONTRACT PLANS OR SPECIFICATIONS ARE TO BE SUPPLIED BY THE CONTRACTOR. ANY JOINT MATERIALS SHOWN AT THE INTERFACE OF PRECAST PANELS AND CAST-IN-PLACE CONCRETE STRUCTURES ARE TO BE SUPPLIED THE ERECTION CONTRACTOR. ALL SANDBLASTING, PAINTING, SEALERS OR OTHER SPECIAL APPLIED COATINGS ARE ALSO SUPPLIED / INSTALLED BY THE CONTRACTOR IN THE FIELD FOLLOWING PANEL ERECTION.

21. FOSTER GEOTECHNICAL SUPPLIES PREFABRICATED WIRE FACING PANELS AND ACCESSORIES TO BE USED IN CONJUNCTION WITH OTHER MATERIALS IN THE CONSTRUCTION OF RETAINED EARTH WALLS DETAILED HEREIN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL FURNISHED BY FOSTER GEOTECHNICAL IS INTENDED TO PROVIDE A GENERAL EXPLANATION OF THE SYSTEM.IT IS THE CONTRACTOR'S OBLIGATION TO DEVISE AND EXECUTE A PROJECT SPECIFIC ERECTION SEQUENCE. PANEL UNLOADING, HANDLING AND BRACING SYSTEM. AND FALL PROTECTION SYSTEM. THE BRACING SYSTEM SHOWN IN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL IS GENERAL IN NATURE AND DOES NOT ACCOUNT FOR PROJECT SPECIFIC CRITERIA COMPLIANCE WITH THE GUIDELINES IN THIS MANUAL DOES NOT RELIEVE THE CONTRACTOR OF ITS RESPONSIBILITY TO ADHERE TO THE PROJECT PLANS. SPECIFICATIONS AND CONTRACT DOCUMENTS OR COMPLIANCE WITH ALL FALL PROTECTION, SAFETY, LAWS, STANDARDS AND PROCEDURES AT THE JOBSITE. CONTRACTORS SHOULD TAKE SPECIAL PRECAUTIONS TO PREVENT THE PANELS FROM SHIFTING OR FAILING DURING THE ERECTION PROCESS.

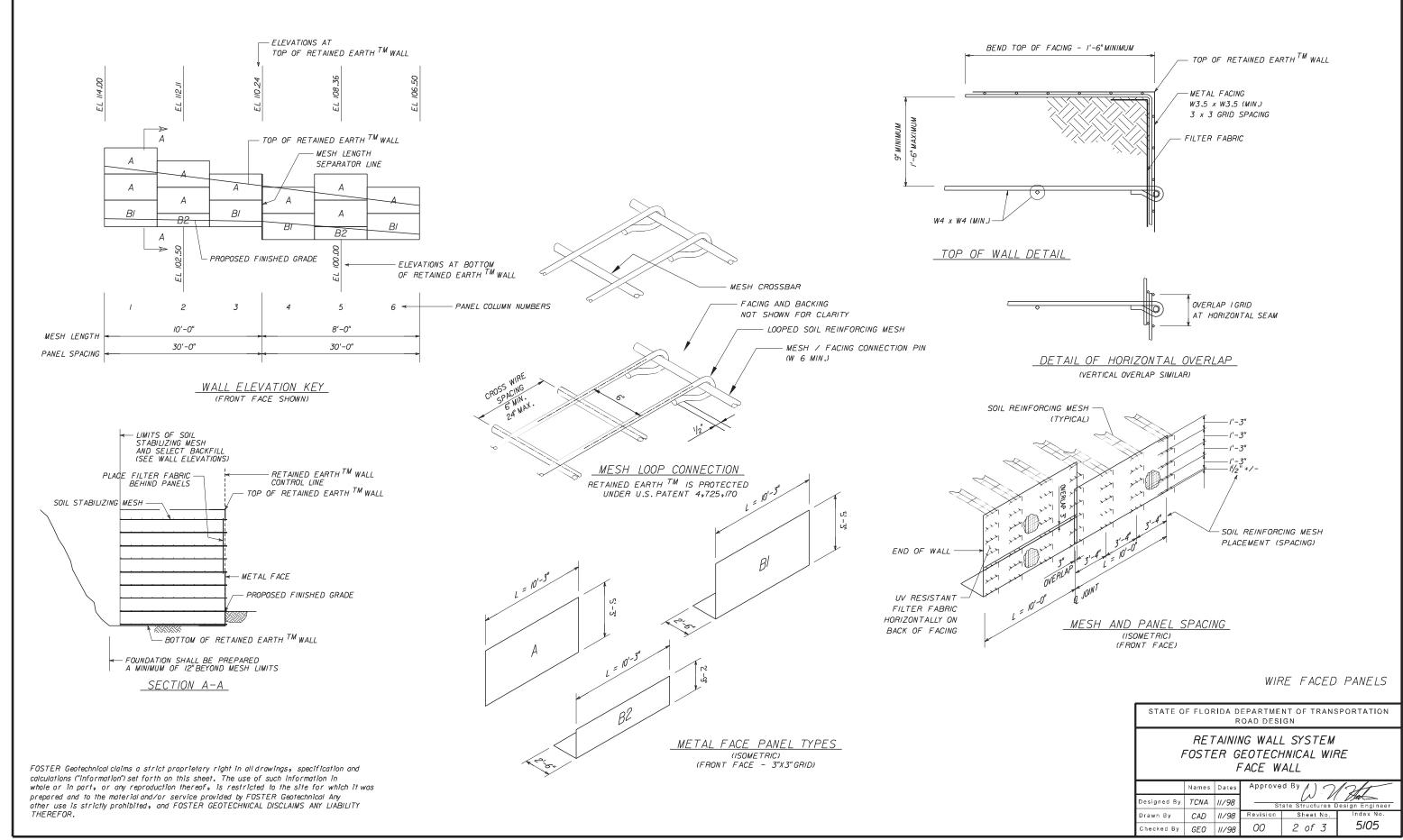
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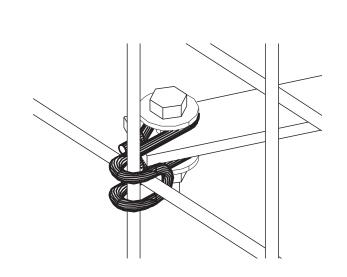
WIRE FACED PANELS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

> RETAINING WALL SYSTEM FOSTER GEOTECHNICAL WIRE FACE WALL

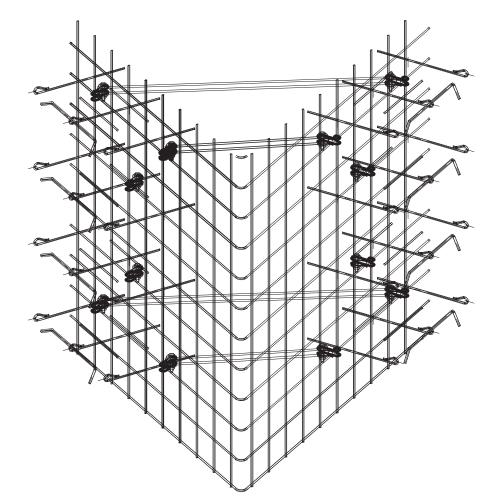
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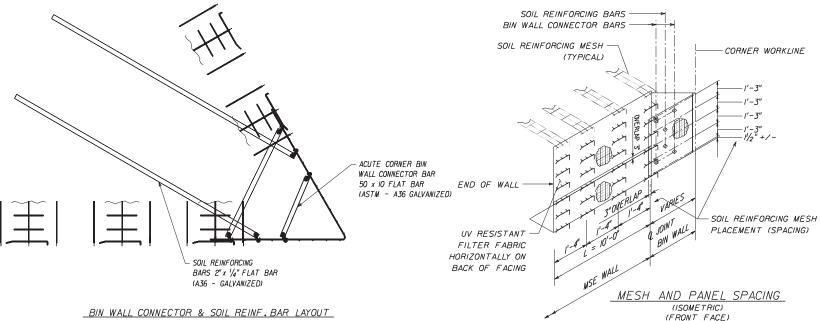


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ISOMETRIC VIEW OF CONNECTION



<u>ISOMETRIC VIEW OF BIN WALL</u>





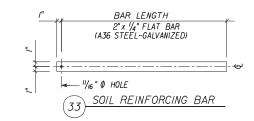
FIELD CUT
TO REQUIRED LENGTH

* EXPOSED STEEL ON FIELD MODIFIED END
SHALL BE COATED WITH ZINC RICH PAINT

FIELD DRILL -"/16" Ø HOLE

- 11/16 " Ø HOLE

32) BIN WALL CONNECTOR BAR



VERTICAL WIRE FACING BAR
= FRONT FACE OF WALL

STATE OF WALL

LELEVATION

HORIZONTAL WIRE FACING BAR (VERTICAL FACING BAR IN FRONT)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM FOSTER GEOTECHNICAL WIRE FACE WALL

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TFRRATRFI

A WIRE FACED MSE WALL SYSTEM

DESIGN CRITERIA

- I. DESIGN IS BASED ON THE ASSUMPTION THAT THE MATERIAL WITHIN, BEHIND, AND BENEATH THE REINFORCED VOLUME; METHODS OF CONSTRUCTION; AND QUALITY OF PREFABRICATED MATERIALS SHALL CONFORM TO SECTION 548.
- 2. SOIL PARAMETERS:

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF FRICTION ANGLE (\$\phi\$), COHESION (\$\cappa\$) AND TOTAL UNIT WEIGHT (Y) SHALL BE PROVIDED IN THE SHOP DRAWINGS.

- 3. THE MAXIMUM APPLIED BEARING PRESSURE AT THE FOUNDATION LEVEL IS AS SHOWN ON THE WALL ELEVATIONS FOR EACH DESIGN CASE. IT IS THE RESPONSIBILITY OF THE ENGINEER TO DETERMINE THAT THIS APPLIED BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED VOLUME, AS DETERMINED BY THE ENGINEER, SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL OR OTHERWISE STABILIZED AS DIRECTED BY THE ENGINEER.
- 5. THE MINIMUM FACTORS OF SAFETY REQUIRED FOR DESIGN

OVERTURNING = 2.0 SLIDING = 1.5INTERNAL PULLOUT = 1.5 (ALLOWABLE DEFORMATION = 0.75 INCH) BEARING CAPACITY = 2.5 OVERALL STABILITY = 1.5 STEEL SOIL REINFORCEMENT = 0.55FV AT END OF DESIGN LIFE AND 0.50 Fu AT NET SECTION OF BOLTED CONNECTION MAXIMUM PULLOUT FACTOR

FOR STRIPS f * (FOR SAND) = 1.5 $(FOR\ LIMEROCK) = 2.0$

FOR LADDERS NoMAX = 30

LAYOUT

6. FOR LOCATION OF THE WALLS, SEE RETAINING WALL CONTROL PLANS.

<u>CONSTRUCTION</u>

- 7. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH SECTION 548. INSTALLATION OF REINFORCING LADDERS SHALL BE PERMITTED ONLY AFTER PLACEMENT AND COMPACTION OF THE BACKFILL MATERIAL HAS REACHED THE REQUIRED LEVEL.
- 8. IF STRUCTURES IN EXCESS OF 20'IN HEIGHT OCCUR, THE FINISHED GRADE IN FRONT OF THE WALL SHALL BE PLACED AND COMPACTED BEFORE WALL CONSTRUCTION EXCEEDS A HEIGHT OF 20', FINISHED GRADE BACKFILL SHALL BE COMPACTED TO 95% OF AASHTO T-180 UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

CONFLICTING STRUCTURES

- 9. IF MANHOLES AND DROP INLETS ARE PRESENT, THEY SHALL BE LOCATED AS SHOWN ON THE WALL ELEVATIONS.
- 10. IF PILES ARE LOCATED WITHIN THE REINFORCED VOLUME, THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE WALL UNLESS A METHOD TO PROTECT THE STRUCTURE, WHICH IS ACCEPTABLE TO THE ENGINEER AND THE REINFORCED EARTH COMPANY, IS PROPOSED AND APPROVED IN WRITING.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE LOCATION OF ANY GUARDRAIL POSTS WITHIN THE REINFORCED VOLUME, PRIOR TO PLACEMENT OF THE TOP LAYERS OF REINFORCEMENTS, INDIVIDUAL REINFORCING LADDERS MAY BE SYSTEMATICALLY SHIFTED TO AVOID THE POST LOCATIONS IF AUTHORIZED BY THE ENGINEER. ANY DAMAGE DONE TO THE REINFORCING ADDERS DUE TO INSTALLATION OF GUARDRAIL POSTS SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- 12. IF EXISTING OR FUTURE STRUCTURES, PIPES, FOUNDATIONS OR GUARDRAIL POSTS WHICH ARE WITHIN THE REINFORCED VOLUME INTERFERE WITH THE NORMAL PLACEMENT OF REINFORCING LADDERS AND SPECIFIC DIRECTION HAS NOT BEEN PROVIDED ON THE PLANS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO DETERMINE WHAT COURSE OF ACTION SHOULD BE TAKEN, UNLESS SHOWN OTHERWISE.
- 13. THE CONTRACTOR IS RESPONSIBLE FOR GRADUALLY DEFLECTING UPPER REINFORCING LADDERS DOWNWARD TO AVOID CONFLICTS WITH PAVING AND SUBGRADE PREPARATION. THE CONTRACTOR'S ATTENTION IS DIRECTED ESPECIALLY TO SITUATIONS WHERE ROADWAY SUPERELEVATION AND/OR SOIL MIXING ARE ANTICIPATED.

MATERIALS NOTES

14. SUPPLIES

ONLY THE FOLLOWING MATERIALS ARE SUPPLIED BY THE REINFORCED EARTH COMPANY:

- PREFABRICATED WIRE FACING PANELS
- WIRE REINFORCING LADDERS OR STRIPS
- HANDLE BAR CONNECTORS OR PINS
- MX4 SOIL RETENTION FABRIC OR EQUAL

ANY OTHER MATERIALS CALLED FOR IN THE CONTRACT PLANS OR SPECIFICATIONS ARE TO BE SUPPLIED BY THE CONTRACTOR.

15. LADDER OR STRIP LENGTH

THE REINFORCING LADDER LENGTHS SHOWN ON THE PLANS ARE MEASURED FROM THE BACK FACE OF THE WIRE FACING PANELS TO THE LIMIT OF THE SELECT BACKFILL MATERIAL, AND ARE THE LENGTHS USED IN THE REINFORCEMENT DESIGN CALCULATIONS.

- 16. THE REINFORCED EARTH COMPANY SUPPLIES FACING PANELS AND ACCESSORIES TO BE USED IN CONJUNCTION WITH OTHER MATERIALS IN THE CONSTRUCTION OF THE REINFORCED EARTH ® RETAINING WALLS DETAILED HEREIN. THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL FURNISHED BY THE REINFORCED EARTH COMPANY IS INTENDED TO PROVIDE A GENERAL EXPLANATION OF THE SYSTEM. IT IS THE CONTRACTOR'S OBLIGATION TO DEVISE AND EXECUTE A PROJECT SPECIFIC ERECTION SEQUENCE, PANEL UNLOADING, HANDLING AND BRACING SYSTEM, AND FALL PROTECTION SYSTEM. THE BRACING SYSTEM SHOWN IN THE CONSTRUCTION AND QUALITY CONTROL PROCEDURES MANUAL IS GENERAL IN NATURE AND DOES NOT ACCOUNT FOR PROJECT SPECIFIC CRITERIA. COMPLIANCE WITH THE GUIDELINES IN THIS MANUAL DOES NOT RELIEVE THE CONTRACTOR OF ITS RESPONSIBILITY TO ADHERE TO THE PROJECT PLANS, SPECIFICATIONS AND CONTRACT DOCUMENTS OR COMPLIANCE WITH ALL FALL PROTECTION, SAFETY, LAWS, STANDARDS AND PROCEDURES AT THE JOBSITE. CONTRACTORS SHOULD TAKE SPECIAL PRECAUTIONS TO PREVENT THE PANELS FROM SHIFTING OR FALLING DURING THE ERECTION PROCESS.
- 17. THE REINFORCED EARTH COMPANY IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.
- 18. THIS DRAWING CONTAINS INFORMATION PROPRIETARY TO THE REINFORCED EARTH COMPANY, AND IS BEING FURNISHED FOR THE USE OF FLORIDA DEPARTMENT OF TRANSPORTATION ONLY IN CONNECTION WITH FDOT PROJECTS, AND THE INFORMATION CONTAINED HEREIN IS NOT TO BE TRANSMITTED TO ANY OTHER ORGANIZATION UNLESS SPECIFICALLY AUTHORIZED IN WRITING BY THE REINFORCED EARTH COMPANY. THE REINFORCED EARTH COMPANY IS EXCLUSIVE LICENSEE IN THE UNITED STATES UNDER PATENTS ISSUED TO HENRY VIDAL, AND THE FURNISHING OF THIS DRAWING DOES NOT CONSTITUTE AN EXPRESSED OR IMPLIED LICENSE UNDER THE VIDAL PATENTS.

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19. THESE DRAWINGS ARE CERTIFIED WITH RESPECT TO THE INTERNAL STABILITY OF REINFORCED EARTH STRUCTURES ONLY

	ROAD DESIGN
TUE	RETAINING WALL SYSTEM

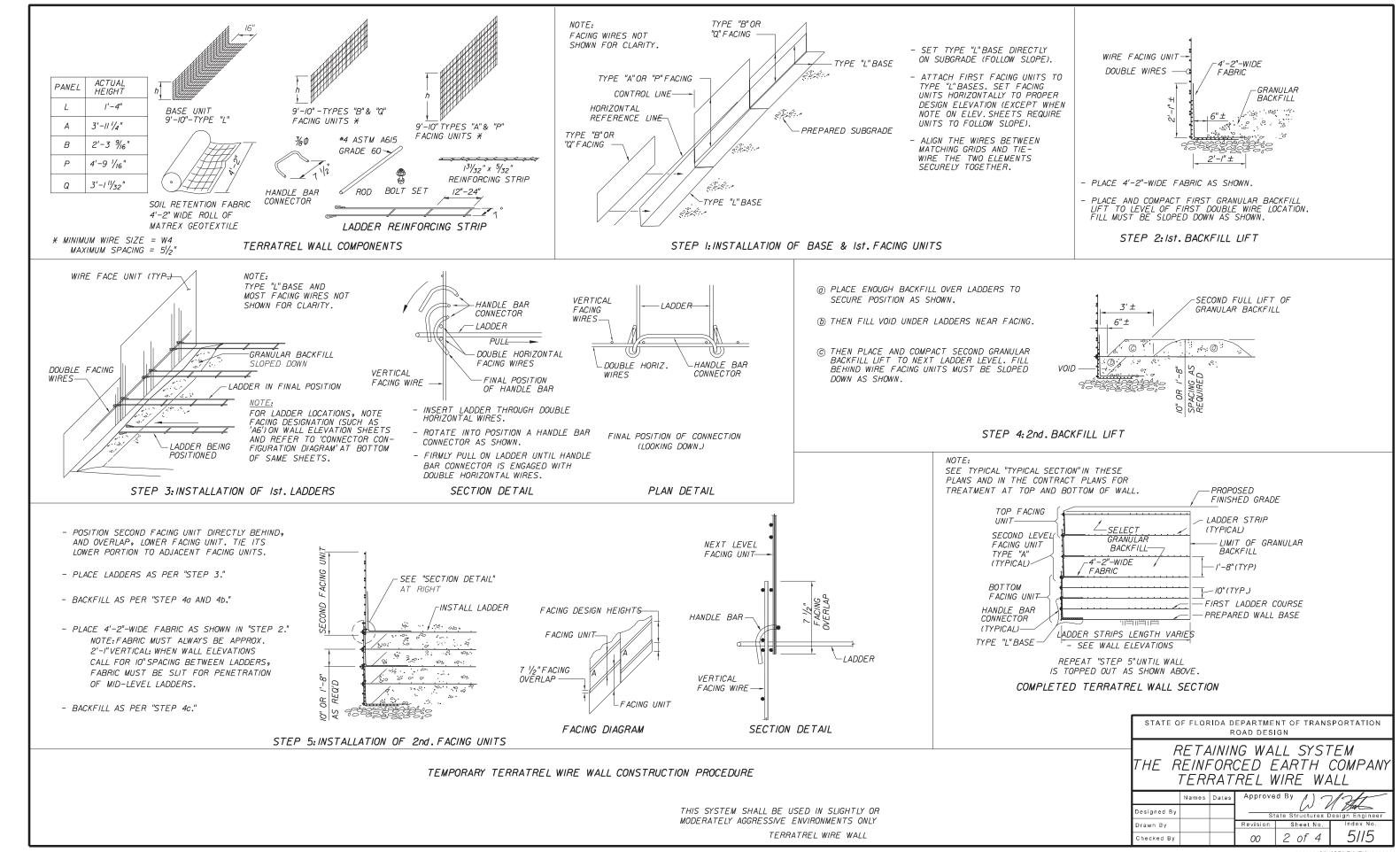
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

TERRATREL WIRE WALL Names Date: Designed By

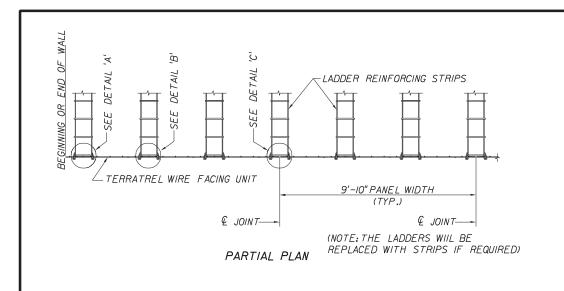
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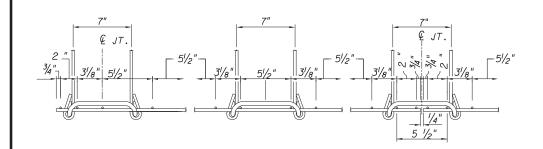
THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY TERRATREL WIRE WALL

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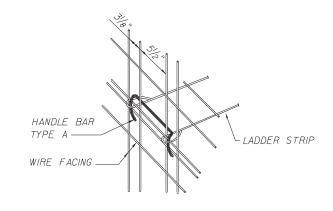


DETAIL 'A'

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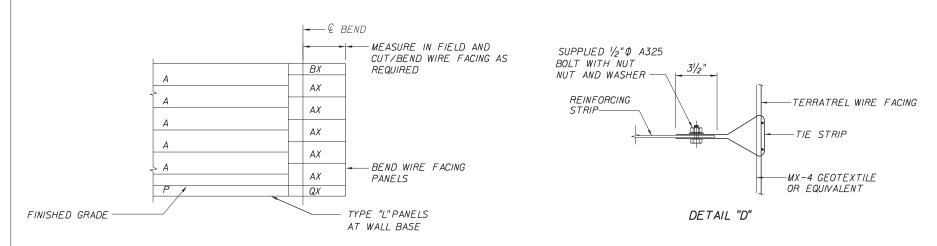
DETAIL 'B'

DETAIL 'C'

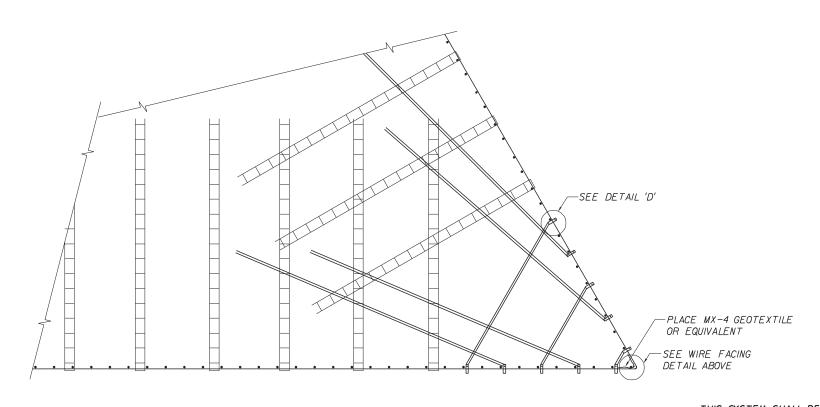


TYPE A HANDLE BAR CONNECTION IN PERSPECTIVE

TERRATREL TYPE A CONNECTION DETAILS



WIRE FACING AT INDSIDE AND OUTSIDE CORNERS



EXAMPLE ACUTE CORNER - SKEWED STRIPS

THIS SYSTEM SHALL BE USED IN SLIGHTLY OR MODERATELY AGGRESSIVE ENVIRONMENTS ONLY

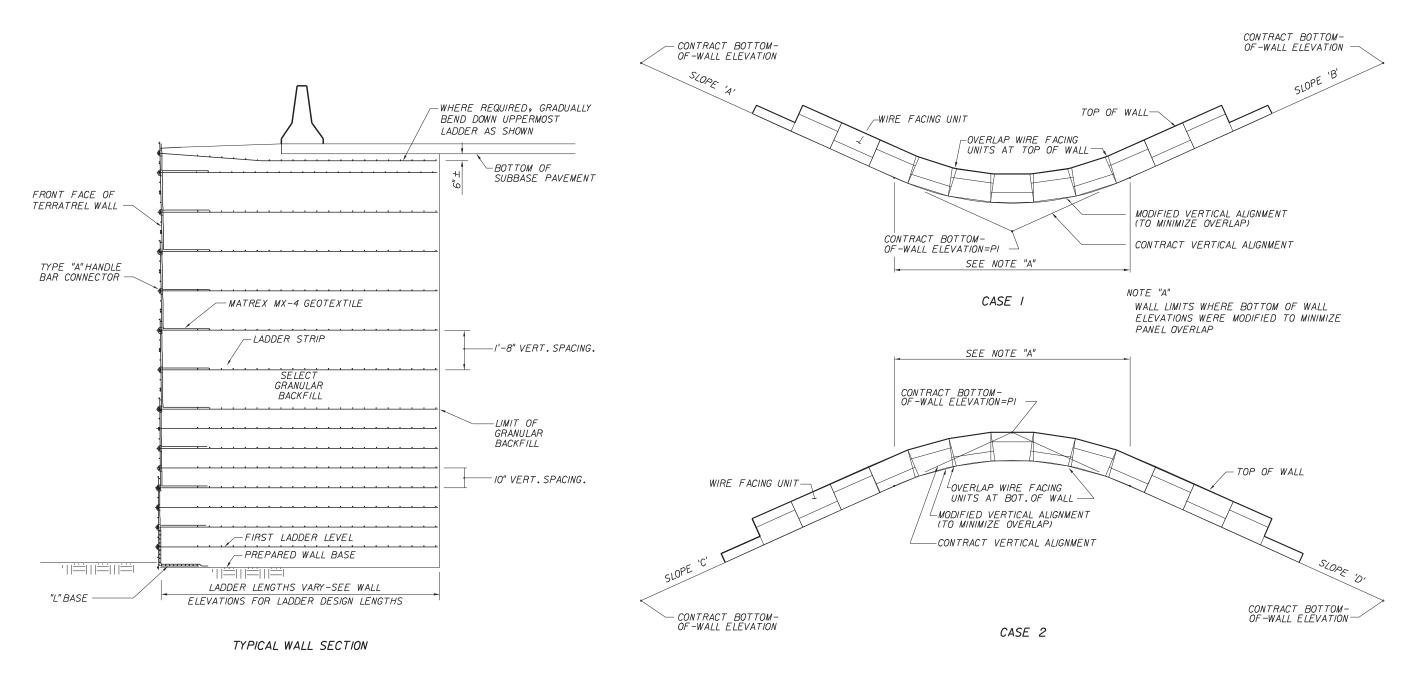
TERRATREL WIRE WALL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM THE REINFORCED EARTH COMPANY TERRATREL WIRE WALL

	Names	Dates	Approve	d By	12/1
Designed By			St	tate Structures E	Design Engineer
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VERTICAL ALIGNMENT DIAGRAMS

(SLOPES HAVE BEEN SHOWN EXAGGERATED FOR CLARITY)
ADDING THE CURVES TO THE VERTICAL ALIGNMENT IS OPTIONAL, AND WHEN USED,
MAY ELIMINATE OVERLAPPING FOR LOW WALLS (IO TO 15 FT.IN HEIGHT) WITH SMALL
CHANGES IN SLOPE UP TO 3%.

RETAINING WALL SYSTEM
THE REINFORCED EARTH COMPANY
TERRATREL WIRE WALL

Names Dates

Designed By
Drawn By
Checked By

Checked By

RETAINING WALL SYSTEM
APPROVED EARTH COMPANY
TERRATREL WIRE WALL

Approved By
State Structures Design Engineer
Drawn By
Checked By

OO 4 of 4 51/5

THIS SYSTEM SHALL BE USED IN ALL ENVIRONMENTS.



HILFIKER MSE WELDED WIRE WALL SYSTEM



GENERAL NOTES

DESIGN CRITERIA

THE ATTACHED DETAILS ARE BASED ON THE ASSUMPTIONS THAT THE MATERIAL WITHIN THE REINFORCED VOLUME, METHODS OF CONSTRUCTION AND QUALITY OF PREFABRICATED COMPONENTS MEET THE GOVERNING AGENCIES SPECIFICATION FOR MECHANICALLY STABILIZED EARTH STRUCTURES

2. MINIMUM DESIGN PARAMETERS

REFERENCE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF THE INTERNAL FRICTION ANGLE ,PHI, THE COHESION, C, AND THE UNIT WEIGHT, GAMMA, SHALL BE PROVIDED IN THE SHOP DRAWINGS.

EXTERNAL STABILITY

OVERTURNING ≥ 2.0 SLIDING ≥ 1.5 BEARING PRESSURE ≥ 2.5

OVERALL STABILITY ≥ 1.5

INTERNAL STABILITY

PULLOUT
STEEL YIELD STRESS = 0.47 F y

SERVICE LIFE = 75 YEARS

LIVE LOAD SURCHARGE = 250 PSF

- 3. THE MAXIMUM APPLIED BEARING PRESSURE AT THE INTERFACE OF THE FOUNDATION AND SELECT BACKFILL MATERIAL IS SHOWN ON THE PLANS. THE BEARING PRESSURE SHOWN IS THE MAXIMUM FOR THE GIVEN BASE MAT LENGTH. IT IS THE RESPONSIBILITY OF OTHERS TO DETERMINE THAT THE BEARING PRESSURE IS ALLOWABLE FOR THAT LOCATION.
- 4. ANY UNSUITABLE FOUNDATION MATERIAL BELOW THE REINFORCED VOLUME AS DETERMINED BY THE ENGINEER SHALL BE EXCAVATED AND REPLACED WITH SUITABLE MATERIAL AS DIRECTED BY THE ENGINEER.
- 5. THE DESIGN CONTAINED ON THESE DRAWINGS ARE BASED ON INFORMATION PROVIDED BY OTHERS, ON THE BASIS OF THIS INFORMATION, T&B STRUCTURAL SYSTEMS IS RESPONSIBLE FOR THE INTERNAL STABILITY OF THE STRUCTURE ONLY, EXTERNAL STABILITY, INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

WALL CONSTRUCTION

- I. WALLS FOUNDED ON CURVES SHALL HAVE THEIR PANELS DIMENSIONED AS A SERIES OF SHORT CORDS (AS DIMENSIONED) IN ORDER TO MATCH THE REQUIRED WALL RADIUS.
- 2. FOR LOCATION AND ALIGNMENT OF THE MSE STRUCTURES REFERENCE THE RETAINING WALL CONTROL PLANS.
- 3. IF MANHOLE AND DROP INLETS ARE REQUIRED, THEY SHALL BE LOCATED AS SHOWN ON THE RETAINING WALL ELEVATION DRAWINGS.
- 4. IF PILES ARE LOCATED WITHIN THE REINFORCED VOLUME THEY SHALL BE DRIVEN PRIOR TO CONSTRUCTION OF THE WALL UNLESS AN ALTERNATE METHOD IS USED TO ISOLATE THE COLUMNS FROM THE REINFORCED VOLUME AS APPROVED BY THE ENGINEER.
- 5. BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WE WITH SECTION 548 TO A LEVEL 2" (PLUS OR MINUS) ABOVE THE ELEVATION OF THE SOIL REINFORCING ELEMENT. NO SOIL REINFORCEMENT SHALL BE ATTACHED TO ANY PANEL BEFORE THE BACKFILL IS PLACED AT THE REQUIRED ELEVATION AND IS COMPACTED.
- 6. STRUCTURES GREATER THAN 20 FEET SHALL HAVE THE FINISHED GRADE PLACED AND COMPACTED AT THE FRONT FACE OF THE STRUCTURE BEFORE THE STRUCTURE HEIGHT EXCEEDS 20 FEET. THE FINISH GRADE SHALL BE COMPACTED TO 95 PERCENT OF AASHTO T-180 UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

- 7. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY GUARDRAIL POSTS PRIOR TO PLACING THE TOP ROW OF SOIL REINFORCEMENT. THE POST SPACING SHALL BE ADJUSTED TO AVOID CONFLICTS WITH THE LONGITUDINAL SOIL REINFORCING WIRE. CUTTING OF THE LONGITUDINAL WIRE SHALL BE ALLOWED ONLY AS DIRECTED BY THE ENGINEER.
- IF EXISTING OR FUTURE STRUCTURES ARE TO BE PLACED IN THE REINFORCED VOLUME THAT INTERFERE WITH THE PROPER PLACEMENT OF THE SOIL REINFORCEMENT THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY FOR A COURSE OF ACTION.
- 9. THE CAP MAT SHALL BE PLACED AS CLOSE TO THE TOP OF WALL LOCATION AS POSSIBLE THE REMAINING FACE PANEL ABOVE THE CAP MAT MAY BE CUT FREE
- O. FOR OTHER INFORMATION PERTAINING TO THE CONSTRUCTION OF THE HILFIKER RETAINING WALL PLEASE REFER TO T&B STRUCTURAL SYSTEMS ERECTION MANUAL.
- II. IT IS THE RESPONSIBILITY OF THE THE CONTRACTOR TO DEFLECT THE TOP CAP MAT OF THE SOIL REINFORCEMENT DOWNWARD SO AS TO NOT CONFLICT WITH ROADWAY MIXING OPERATIONS AND/OR ROADWAY CONSTRUCTION OPERATIONS, ANY SOIL REINFORCING MATERIAL THAT IS DAMAGED SHALL BE REPLACED AT THE CONTRACTORS EXPENSE.

CONSTRUCTION NOTES

I. NOMINAL SOIL REINFORCING GRID LENGTH

THE WELDED WIRE MESH IS MANUFACTURED IN LENGTHS CORRESPONDING TO THE DIMENSION "B" AS GIVEN IN THE RETAINING WALL ELEVATIONS. THE ACTUAL LENGTH FROM THE FRONT FACE OF THE PANEL TO THE TAIL OF THE SOIL REINFORCING GRID IS PLUS 2"-4". THE FOUNDATION SHALL BE EXCAVATED TO AN EXTENT OF "B" PLUS 6".

- 2 THE FOLLOWING MATERIALS ARE SUPPLIED BY T&B STRUCTURAL SYSTEMS
 - . WELDED WIRE FACING PANEL
 - SOIL REINFORCING GRIDS
 - · CAP MATS
 - CONNECTION PINS
 - SYNTHETIC INDUSTRIES GEOTEX 401 NONWOVEN GEOTEXTILE FILTER FABRIC

ANY OTHER MATERIAL REQUIRED TO BUILD THE MSE STRUCTURES ACCORDING TO THE GOVERNING SPECIFICATION SHALL BE SUPPLIED BY THE CONTRACTOR.

3. T&B STRUCTURAL SYSTEM SUPPLIES MECHANICALLY STABILIZED EARTH STRUCTURAL COMPONENTS FOR USE WITH THE HILFIKER RETAINING WALL SYSTEMS FOR THE STRUCTURES DETAILED HEREIN. THE ERECTION MANUAL PROVIDED BY T&B STRUCTURAL SYSTEMS IS A GENERAL GUIDELINE FOR ERECTING THE HILFIKER RETAINING WALL SYSTEM. ALL QUALITY CONTROL PROCEDURES, STAGING PROCEDURES, MATERIAL HANDLING, AND SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. THIS DOES NOT RELIEVE THE CONTRACTOR OF THE OBLIGATION TO CONSTRUCT THE RETAINING WALL ACCORDING TO THE PROJECT PLANS AND SPECIFICATIONS AND ALL LAWS OF THE GOVERNING STATE.

ENGL/SH

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM HILFIKER WELDED WIRE WALL

	Names	Dates	Approve	d By /) 7	12/		
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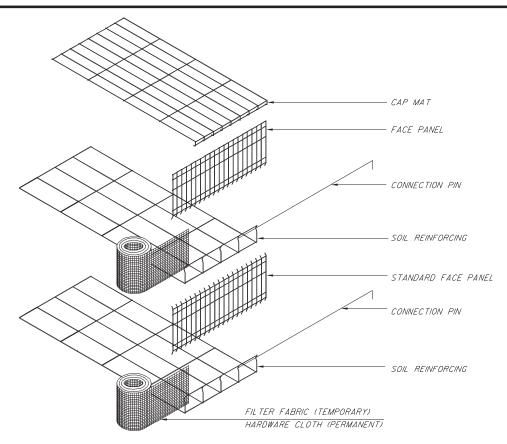
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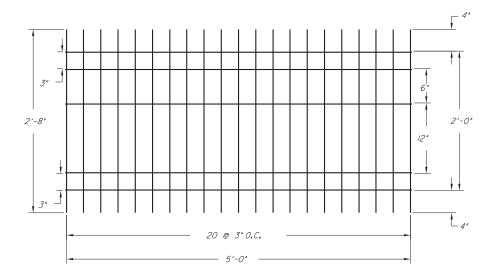
188 STRUCTURAL SYSTEMS INC.

ENGINEERED STRUCTURES
637 WEST HURST BLVD.
HURST, TEXAS 76053
888-280-9858

HILFIKER RETAINING WALL



A WELDED WIRE WALL COMPONENT ISOMETRIC



FRONT VIEW

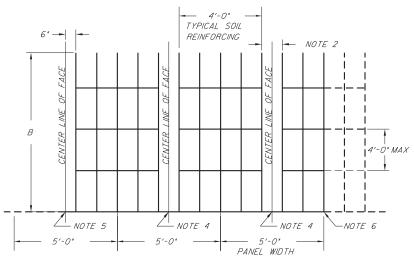


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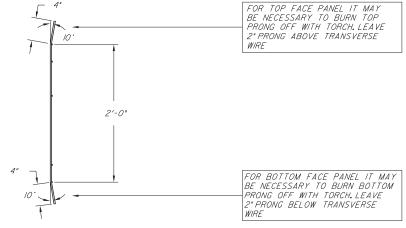
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NOTE:

- I. SOIL REINFORCING MAT TO BE PLACED ON PREPARED SURFACE
- 2. 12" SPACE BETWEEN SOIL REINFORCING MAT U.N.O.
- 3. PLACE FACE PANEL AT MIDPOINT OF SOIL REINFORCING MAT
- 4. BUTT FACE PANEL TOGETHER AND SECURE WITH A HOG RING
- 5. AT START OF WALL PLACE SOIL MAT AND TRIM EXCESS FACE PANEL
- 6. AT END OF WALL PLACE SOIL MAT AND FACE PANEL AND TRIM EXCESS





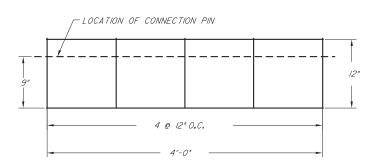
NOTE

- I. BOTTOM FACE PANEL MAY NEED TO HAVE PRONGS BURNED OFF IN FIELD
- 2. BURN PRONGS OFF 2" FROM TRANSVERSE WIRE
- 3. GALVANIZED FACE PANELS REQUIRE EXPOSED BLACK STEEL TO BE COATED WITH RICH ZINC PAINT OR APPROVED EQUAL
- 4. INTERSECTION OF ADJACENT FACE PANEL SECURE VERTICAL WIRES TOGETHER AT INTERFACE

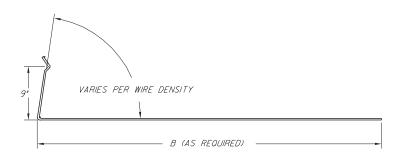


FACE PANEL SECTION

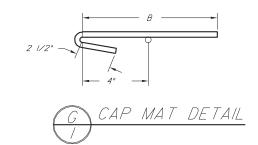












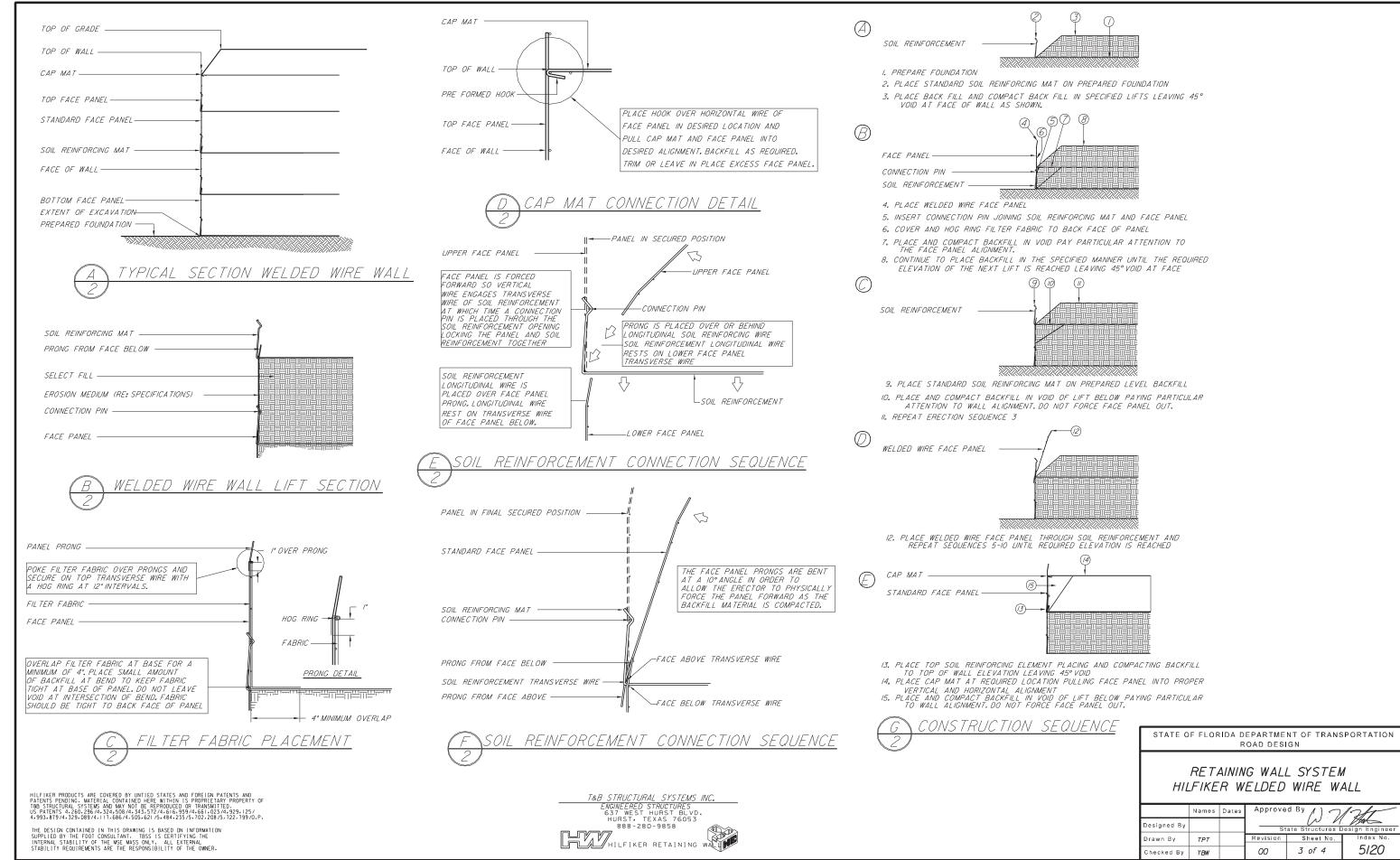


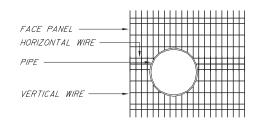


STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM HILFIKER WELDED WIRE WALL

	Names	Dates	Approve	d By /) 7/	12/1	
Designed By			State Structures Design Engineer			
Drawn By	TPT		Revision	Sheet No.	Index No.	
Checked By	TBW		00	2 of 4	<i>5120</i>	





NOTE:
TRIM PROTRUSION AREA FROM FACE PANEL BY CUTTING
HORIZONTAL WIRE BETWEEN EACH VERTICAL WIRE. BEND
WIRES BACK INTO MSE MASS AND AS CLOSE TO PROTRUSION
AS POSSIBLE. APPLY FILTER FABRIC OVER AND AROUND
PROTRUSION MAKING SURE FACE PANEL IS COVERED. MAKE
SURE THAT ALL GAPS BETWEEN FACE AND PROTRUSION ARE
COVERED WITH FILTER FABRIC. IF PROTRUSION INTERFERES
WITH SOIL REINFORCING MAT CUT TRANSVERSE WIRES OF MAT
AND BEND LONGITUDINAL WIRE TO PASS PROTRUSION AND
CONFORM TO THE PROTRUSIONS SHAPE.

FACE OF WALL STANDARD FACE PANEL TRIMMED AND BENT FACE PANEL TRIMMED SOIL REINFORCING MAT PROTRUSION FILTER FABRIC SOIL REINFORCING MAT

NOTE:
TRIM PROTRUSION AREA FROM FACE PANEL BY CUTTING
HORIZONTAL WIRE BETWEEN EACH VERTICAL WIRE, BEND
WIRES BACK INTO MSE MASS AND AS CLOSE TO PROTRUSION
AS POSSIBLE. APPLY FILTER FABRIC OVER AND AROUND
PROTRUSION MAKING SURE FACE PANEL IS COVERED. MAKE
SURE THAT ALL CAPS BETWEEN FACE AND PROTRUSION ARE
COVERED WITH FILTER FABRIC, IF PROTRUSION INTERFERES
WITH SOIL REINFORCING MAT CUT TRANSVERSE WIRES OF MAT
AND BEND LONGITUDINAL WIRE TO PASS PROTRUSION AND
CONFORM TO THE PROTRUSIONS SHAPE.

STANDARD FACE PANEL TRIMMED AND BENT FACE PANEL FILTER FABRIC SOIL REINFORCING MAT

NOTE:
TRIM PROTRUSION AREA FROM FACE PANEL BY CUTTING
HORIZONTAL WIRE BETWEEN EACH VERTICAL WIRE BEND
WIRES BACK INTO MSE MASS AND AS CLOSE TO PROTRUSION
AS POSSIBLE, APPLY FILTER FABRIC OVER AND AROUND
PROTRUSION MAKING SURE FACE PANEL IS COVERED. MAKE
SURE THAT ALL GAPS BETWEEN FACE AND PROTRUSION ARE
COVERED WITH FILTER FABRIC, IF PROTRUSION INTERFERES
WITH SOIL REINFORCING MAT CUT TRANSVERSE WIRES OF MAT
AND BEND LONGITUDINAL WIRE TO PASS PROTRUSION AND
CONFORM TO THE PROTRUSIONS SHAPE.

A TYPICAL ELEVATION THROUGH PENETRATION GABION BIN WALL STANDARD PANEL SOIL REINFORCEMENT FRONT FACE OF WALL BIN WALL BIN WALL WIT SPIRAL BINDER NOSE SECTION BENT TO REQUIRED

NOTE: 12 GAUGE GALVAINZED STEEL HOG RING MAY BE SUBSTITUDED FOR SPIRAL BINDER. HOG RINGS TO BE ATTACHED AT 3" CENTERS TOP TO BOTTOM.

ANGLE (ONE PIECE)

ISOMETRIC OF BIN GABION NOSE SECTION

YPICAL PLAN VIEW THROUGH PENETRATION 45 REO'D-TEMP WIRE WALL VARIES SPIRAL BIND PANEL TO PRE-BENT NOSE SECTION TYPICAL ALL INTERSECTIONS ATTACH STANDARD SOIL REINFORCING MAT TO BACK OF GABION BIN WALL INTERSECTING WALL FACE INTERMEDIATE DIAPHRAGM (WHEN APPLICABLE) VARIES VARIES VARIES SPIRAL BIND DIAPHRAGM TO PRE-BENT NOSE SECTION TYPICAL ALL INTERSECTIONS FIELD BEND NOSE PANEL

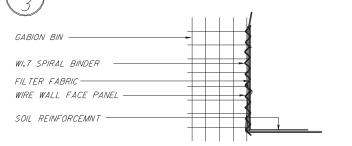




FACE OF WALL

6" FILL TO BE PLACED BETWEEN
SOIL REINFORCING MAT AND OBSTRUCTION

DEFLECT SOIL REINFORCING MAT
BY OBSTRUCTION. MAKE SURE THAT
ALIGNMENT OF FACE PANEL IS
CONSISTENT WITH FACE PANEL BELOW.
DO NOT CUT SOIL REINFORCING MAT.
SOIL REINFORCING MAT MAY BE MOVED UP
ON FACE PANEL TO KEEP DEFLECTION
ANGLE TO A MAXIMUM OF 15^



SECTION AT PARALLEL OBSTRUCTION

NOTE: 12 GAUGE GALVAINZED STEEL HOG RING MAY BE SUBSTITUDED FOR SPIRAL BINDER, HOG RINGS TO BE ATTACHED AT 3° CENTERS



STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM HILFIKER WELDED WIRE WALL

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	Designed By			S1	ate Structures D	Design Engineer
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\$\$\$\$\$\$\$DGNSPECIFICATION\$\$\$\$\$\$\$

CONSTRUCTION NOTES FOR THE PLACEMENT OF TENSAR GEOGRIDS AND BACKFILL SOILS FOR TENSAR WWF TEMPORARY RETAINING WALL

I.O MATERIALS

- I.I GEOGRID REINFORCING SHALL BE TENSAR UNIAXIAL GEOGRID AND BIAXIAL GEOGRIDS MANUFACTURED BY THE TENSAR CORPORATION. MORROW. GEORGIA.
- 1.2 BODKIN BARS SHALL BE 1/2"x/4" HDPE BARS MANUFACTURED BY THE TENSAR CORPORATION, MORROW. GFORGIA.
- I.3 GEOTEXTILE FILTER FABRIC TG600 SHALL BE MANUFACTURED BY EVERGREEN TECHNOLOGIES, INC., EVERGREEN, ALABAMA OR EQUIVALENT AS APPROVED BY THE FNGINFER
- 1.4 WALL FACING
- I.4.I FACING SHALL BE PRE-FABRICATED BLACK STEEL WELDED WIRE FORMS (WWF) 4" x 4" W4.0 x W4.0 AND GEOTEXTILE FABRIC. WIRE FORM GEOMETRY SHALL BE AS DETAILED IN THE CONSTRUCTION DRAWINGS.
- 2.0 TECHNICAL REQUIREMENTS
- 2.1 FILL MATERIALS SHALL BE PLACED FROM THE BACK OF THE WELDED WIRE FACING FORMS TOWARDS THE ENDS OF THE GEOGRID TO ENSURE FURTHER TENSIONING.
- 2.2 WELDED WIRE FACING SHALL BE MONITORED DURING FILL PLACEMENT AND COMPACTION. COMPACTION EQUIPMENT AND OPERATION PROCEDURES MAY HAVE TO BE MODIFIED TO PREVENT EXCESSIVE DEFORMATION OF THE FLEXIBLE WELDED WIRE FACING.
- 2.3 TIE WIRES OR HOG RINGS MAY BE REQUIRED IF WWF MOVES DURING BACKFILL OPERATIONS.
- 3.0 TENSAR GEOGRID PLACEMENT
- 3.1 TENSAR GEOGRID SHALL BE PLACED AT THE SAME LOCATIONS AND ELEVATIONS SHOWN ON THE SHOP DRAWINGS.
- 3.2 TENSAR GEOGRID REINFORCEMENT SHALL BE CONTINUOUS THROUGHOUT THEIR EMBEDMENT LENGTH(S). THE BODKIN CONNECTION SHALL NOT BE UTILIZED UNLESS PRE-APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION.
- 3.2.1 IF PRE-APPROVED, TENSAR UNIAXIAL GEOGRIDS MAY BE SPLICED UTILIZING THE BODKIN CONNECTION DETAIL. NO MORE THAN ONE SPLICE SHALL BE ALLOWED IN ANY ONE LENGTH OF REINFORCING AND NO SPLICES SHALL BE ALLOWED FOR GEOGRIDS LESS THAN 6 feet IN LENGTH (EACH). THE BODKIN CONNECTION SHALL NOT BE PLACED LESS THAN 6 feet BELOW PLANNED FINISHED GRADE, NOR HORIZONTALLY NOR VERTICALLY ADJACENT TO ANOTHER BODKIN CONNECTION.

THIS DESIGN IS BASED UPON SPECIFIC PROPERTIES OF TENSAR PRODUCTS (GEOGRIDS, DRAINAGE COMPOSITES AND EROSION MEDIA), WHICH ARE PROPRIETARY TO THE TENSAR CORPORATION 1210 CITIZENS PARKWAY, MORROW GA. 30260. ANY SUBSTITUTION OF THE SPECIFIED PRODUCTS WILL INVALIDATE THIS DESIGN.

THIS DRAWING, DESIGN NOTES AND ASSOCIATED CALCULATIONS HAVE BEEN PREPARED BY TENSAR EARTH TECHNOLOGIES, INC. FOR PRELIMINARY DESIGN PURPOSES AND SHALL NOT BE USED FOR FINAL DESIGN OR CONSTRUCTION.

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- 3.3 PRIOR TO PLACING FILL, THE GEOGRID MATERIALS SHALL BE PLACED TO LAY FLAT AND PULLED TAUT TO REMOVE ANY SLACK IN THE GEOGRIDS.
- 3.4 TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID. A MINIMUM FILL THICKNESS OF 6 Inches IS REQUIRED FOR OPERATION OF TRACKED VEHICLES OVER THE GEOGRID. TURNING OF TRACKED VEHICLES SHOULD BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND/OR THE GEOGRID.
- 3.5 RUBBER-TIRED VEHICLES MAY PASS OVER THE GEOGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE AVOIDED.
- 3.6 TENSAR UNIAXIAL GEOGRIDS SHALL BE ROLLED OUT WITH THE LONG AXIS OF THE APERTURES (MACHINE DIRECTION) PERPENDICULAR TO THE WELDED WIRE FORM FACE. TENSAR BIAXIAL GEOGRIDS SHALL BE ROLLED OUT WITH THE MACHINE DIRECTION BAR PARALLEL TO THE WELDED WIRE FORM FACE.
- 3.6.1 UNIAXIAL (UX) GEOGRIDS SHALL BE CUT NEXT TO THE CROSS MACHINE DIRECTION BAR. UX GEOGRIDS SHALL BE UNROLLED PERPENDICULAR TO THE WALL FACE.
- 3.6.2 BIAXIAL GEOGRIDS SHALL BE CUT NEXT TO THE MACHINE DIRECTION BAR. BX GEOGRIDS SHALL BE UNROLLED PARALEL TO THE WALL FACE
- 3.7 GEOGRIDS SHALL BE CUT AND PLACED SO THAT A TRANSVERSE BAR IS EXTENDED TO THE BACK FACE OF THE WELDED WIRE FORM.
- 3.8 A MINIMUM OF 3 inches OF FILL MATERIAL SHALL BE REQUIRED BETWEEN LAYERS OF BIAXIAL, UNIAXIAL AND FILTER FABRIC, UNLESS OTHERWISE SHOWN.
- 4.0 CHANGES TO GEOGRID LAYOUT OR PLACEMENT
- 4.1 NO CHANGES TO THE TENSAR GEOGRID LAYOUT, INCLUDING, BUT NOT LIMITED TO, LENGTH, GEOGRID TYPE, OR ELEVATION, SHALL BE MADE WITHOUT THE EXPRESSED PRIOR WRITTEN CONSENT OF TENSAR EARTH TECHNOLOGIES, INC.
- 5.0 DRAINAGE
- 5.1 THE TENSAR REINFORCED WALL HAS BEEN DESIGNED ON THE ASSUMPTION THAT THE REINFORCED BACKFILL MATERIAL SHALL BE FREE OF SUBSURFACE DRAINAGE OF WATER (SEEPAGE).

- 5.0 DESIGN PARAMETERS
- 6,1 SOIL PARAMETERS

SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUES OF FRICTION ANGLE, APPARENT COHESION AND UNIT WEIGHT SHALL BE PROVIDED IN THE SHOP DRAWINGS.

6.1.1 DESIGN

THE DESIGN CONTAINED ON THESE DRAWINGS IS BASED ON INFORMATION PROVIDED BY OTHERS. ON THE BASIS OF THIS INFORMATION, THE TENSAR CORPORATION IS RESPONSIBLE FOR INTERNAL STABILITY OF THE STRUCTURE ONLY. EXTERNAL STABILITY DESIGN INCLUDING FOUNDATION AND SLOPE STABILITY IS THE RESPONSIBILITY OF OTHERS.

- 6.2 FACTORS OF SAFETY:
 - INTERNAL STABILITY:

 MAXIMUM GEOGRID DESIGN STRENGTH = 0.29 ULT

 MINIMUM FACTOR OF SAFETY FOR = 1.5

 GEOGRID PULLOUT

 MINIMUM FACTOR OF SAFETY FOR SLIDING = 1.5

 AT LOWEST GEOGRID

 GEOGRID-SOIL INTERACTION COEFFICIENT = 0.8

 PERCENT COVERAGE OF GEOGRID = VARIES
- 6.2.2 EXTERNAL STABILITY:

 MINIMUM FACTOR OF SAFETY FOR SLIDING = 1.5

 MINIMUM FACTOR FOR SAFETY FOR OVERTURNING = 2.0

 EXTERNAL STABILITY IS THE RESPONSIBILITY OF OTHERS.

 TENSAR EARTH TECHNOLOGIES, INC. ACCEPTS NO LIABILITY OR

RESPONSIBILITY FOR GLOBAL STABILITY. (SEE SECTION 7.5)

- 6.2.3 GLOBAL STABILITY:
 GLOBAL STABILITY IS THE RESPONSIBILITY OF OTHERS.
 TENSAR EARTH TECHNOLOGIES, INC. ACCEPTS NO LIABILITY OR
 RESPONSIBILITY FOR GLOBAL STABILITY. (SEE SECTION 7.5)
- 7.0 SPECIAL PROVISIONS
- 7.1 WALL ELEVATION VIEWS AND LOCATIONS AND GEOMETRY OF EXISTING STRUCTURES MUST BE VERIFIED BY THE CONTRACTOR BEFORE COMMENCEMENT OF SHOP DRAWINGS.
- 7.2 TENSAR EARTH TECHNOLOGIES, INC. ASSUMES NO LIABILITY FOR INTERPRETATION OR VERIFICATION OF SUBSURFACE CONDITIONS, SUITABILITY OF SOIL DESIGN PARAMETERS AND INTERPRETATION OF SUBSURFACE GROUNDWATER CONDITIONS.

- 7.3 ANY REVISIONS TO DESIGN PARAMETERS STATED ON CONTROL DRAWINGS OR STRUCTURE GEOMETRY SHALL REQUIRE DESIGN MODIFICATIONS PRIOR TO PROCEEDING WITH CONSTRUCTION.
- 7.4 THIS DESIGN IS ONLY VALID FOR THE INTERNAL STABILITY OF THE PROPOSED TENSAR REINFORCED RETAINING WALLS AS SHOWN HEREIN.
- 7.5 BEARING CAPACITY, TOTAL SETTLEMENT,
 DIFFERENTIAL SETTLEMENT, AND THEIR EFFECTS ON
 THE TENSAR REINFORCED RETAINING WALL SYSTEM
 SHALL BE THE RESPONSIBILITY OF OTHERS

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM
TENSAR EARTH TECHNOLOGIES
TEMPORARY RETAINING WALL

Designed By

Drawn By

Checked By

Dates

Approved By

State Structures Design Engineer

Revision Sheet No. Index No.

OO 1 of 3 5125

TENSAR

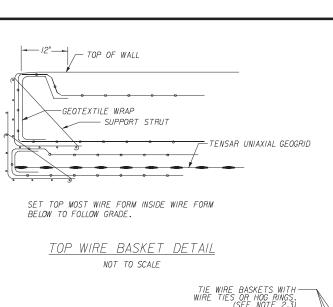
EARTH TECHNOLOGIES, INC.

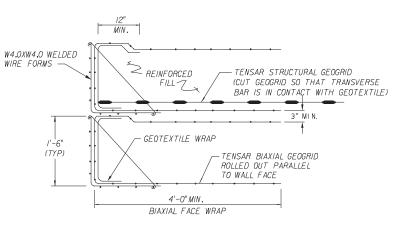
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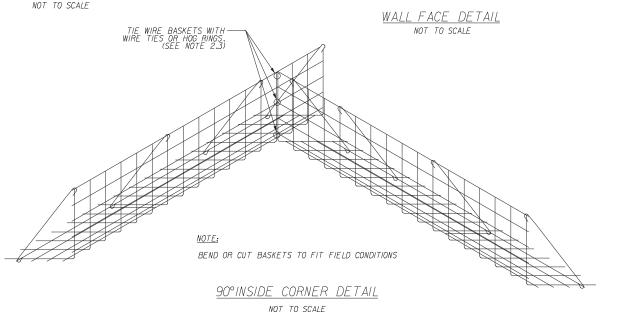
Lakeside Center Sulte 450

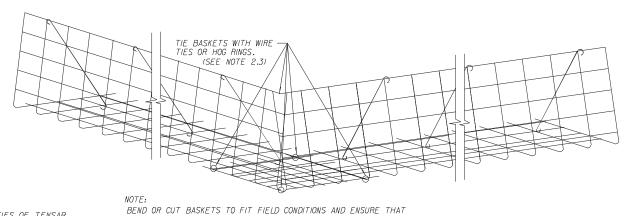
Atlanta, GA 30328

(404) 250-1290









THIS DESIGN IS BASED UPON SPECIFIC PROPERTIES OF TENSAR PRODUCTS (GEOGRIDS, DRAINAGE COMPOSITES AND EROSION MEDIA), WHICH ARE PROPRIETARY TO THE TENSAR CORPORATION 1210 CITIZENS PARKWAY, MORROW GA. 30260. ANY SUBSTITUTION OF THE SPECIFIED PRODUCTS WILL INVALIDATE THIS DESIGN.

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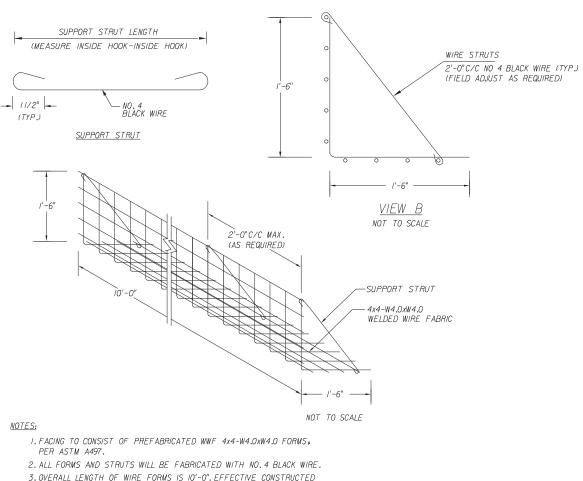
90°OUTSIDE CORNER DETAIL NOT TO SCALE

GEOTEXTILE FILTER FABRIC TG600 AND BIAXIAL GEOGRID OVERLAP I'-O" MINIMUM.

TENSAR

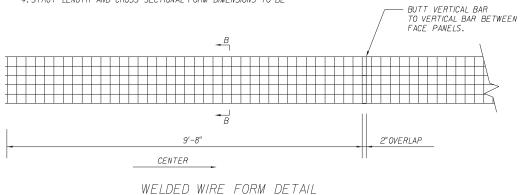
5775-B Gienridge Drive Lakeside Center Suite 450 Atlanta, GA 30328 (404) 250-1290





3. OVERALL LENGTH OF WIRE FORMS IS 10'-O". EFFECTIVE CONSTRUCTED WIDTH IS 9'-8"WITH 2"OVER LAPPING AT ENDS.

4. STRUT LENGTH AND CROSS-SECTIONAL FORM DIMENSIONS TO BE



NOT TO SCALE

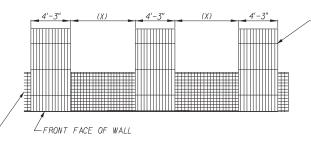
THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

RETAINING WALL SYSTEM TENSAR EARTH TECHNOLOGIES TEMPORARY RETAINING WALL

L		Names	Dates	Approve		12/
D	esigned By			S1	ate Structures D	Design Engineer
D	rawn By	JMS	8/14/98	Revision	Sheet No.	Index No.
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PRIMARY UNIAXIAL GEOGRID

<i>PERCENT</i> COVERAGE	Χ
100	0
75	1'-5"
56	3'-4"

BIAXIAL (1/3 ROLL WIDTH) 4'-3"WIDE ROLLED OUT PARALLEL
TO WALL FACE, BIAXIAL GEOGRID SHALL BE PROVIDED BETWEEN PRIMARY
REINFORCEMENT ONLY WHEN 56% COVERAGE IS SPECIFIED.

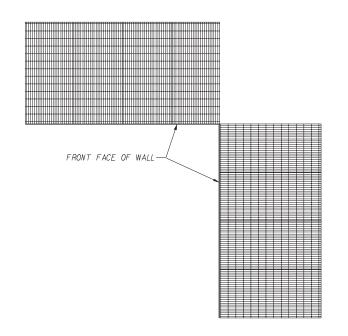
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ALTERNATE LAYERS OF UNIAXIAL PRIMARY REINFORCEMENT SHALL BE PLACED IN STAGGERED PATTERN SUCH THAT THE LAYER ABOVE IS PLACED WITH THE CENTERLINE OF THE GEOGRID IN ALIGNMENT WITH THE CENTERLINE OF THE SPACE BELOW.

TYPICAL GEOGRID COVERAGE NOT TO SCALE

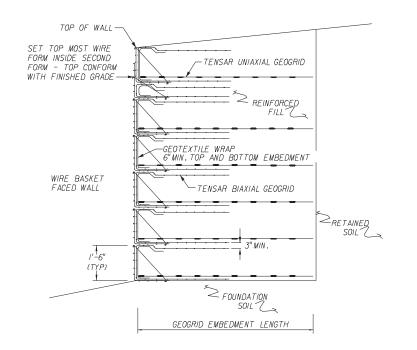


GEOGRID 90°INSIDE CORNER DETAIL
NOT TO SCALE

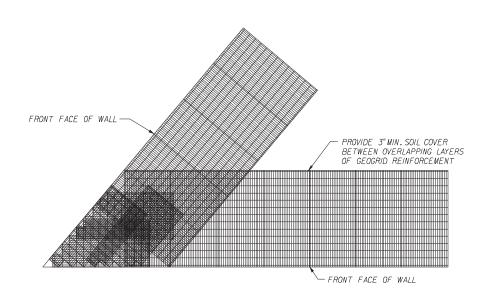
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TYPICAL CROSS-SECTION
NOT TO SCALE

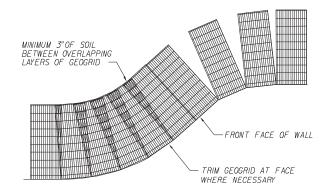


GEOGRID ACUTE CORNER DETAIL

NOT TO SCALE

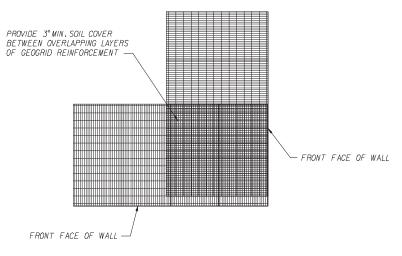






GEOGRID PLACEMENT ON CURVES

NOT TO SCALE



GEOGRID 90°OUTSIDE CORNER DETAIL

NOT TO SCALE

THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
ROAD DESIGN

RETAINING WALL SYSTEM
TENSAR EARTH TECHNOLOGIES
TEMPORARY RETAINING WALL

	Names	Dates	Approve	d By /) _	12/1		
Designed By			State Structures Design Engineer				
Drawn By	JMS	8/14/98	Revision	Sheet No.	Index No.		
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TC Mirafi Engineering Services, Inc.

365 SOUTH HOLLAND DRIVE, PENDERGRASS, GA 30567 TEL (706) 693-2226

CONSTRUCTION NOTES FOR THE PLACEMENT OF MIRAFIREINFORCEMENT AND BACKFILL SOILS FOR TEMPORARY MECHANICALLY STABILIZED EARTH (MSE) WALLS

I.O DESIGN CRITERIA

I.I SOIL PARAMETERS: SEE WALL CONTROL DRAWINGS FOR SOIL CHARACTERISTICS OF FOUNDATION MATERIAL TO BE USED IN THE DESIGN OF THE WALL SYSTEM. THE CONTRACTOR SHALL PROVIDE SOIL DESIGN PARAMETERS FOR BACKFILL MATERIAL BASED ON THE ACTUAL SOIL CHARACTERISTICS UTILIZED AT THE SITE. THE VALUE OF Ø, C, AND SHALL BE PROVIDED IN THE SHOP DRAWINGS.

1.2 MINIMUM FACTOR OF SAFETY

1.2.1 EXTERNAL STABILITY

SLIDING OVERTURNING 2.0 BEARING CAPACITY

1.2.2 INTERNAL STABILITY

RUPTURE **PULLOUT**

1.2.3 GLOBAL STABILITY 1.5

1.2.4 UNIFORM SURCHARGE 250 PSF

1.2.5 HYDROSTATIC FORCES NONF

1.2.6 SEISMIC FORCES

IN ACCORDANCE WITH AASHTO AND FDOT PLANS PREPARATION MANUAL.

2.0 MATERIALS

- 2,1 GEOSYNTHETIC REINFORCEMENT AND RETENTION FABRIC, MIRAFI 140N, SHALL BE MANUFACTURED BY TC MIRAFI, PENDERGRASS, GEORGIA.
- 2.2 REINFORCED BACKFILL SHALL MEET THE REQUIREMENTS IN FLORIDA DOT SPECIFICATIONS - SECTION 548 RETAINING WALL SYSTEMS.
- 2.3 WALL FACING SHALL BE PRE-FABRICATED STEEL WIRE FORMS COMPOSED OF A MINIMUM W3.5 SIZE STANDARD WIRE WELDED ORTHOGONALLY 4 INCHES ON CENTER, STEEL WIRE FORMS SHALL BE AS DETAILED IN THE DRAWINGS.
- 2.4 RING FASTENER SHALL BE BLAIR STYLE #3-LOXIT, IO GAUGE GALVANIZED, MANUFACTURED BY DECKER MANUFACTURING CO.OR EQUIVALENT.

TC Mirafi Engineering Services, Inc. 365 SOUTH HOLLAND DRIVE PENDERGRASS, GEORGIA 30567

3.0 WALL CONSTRUCTION

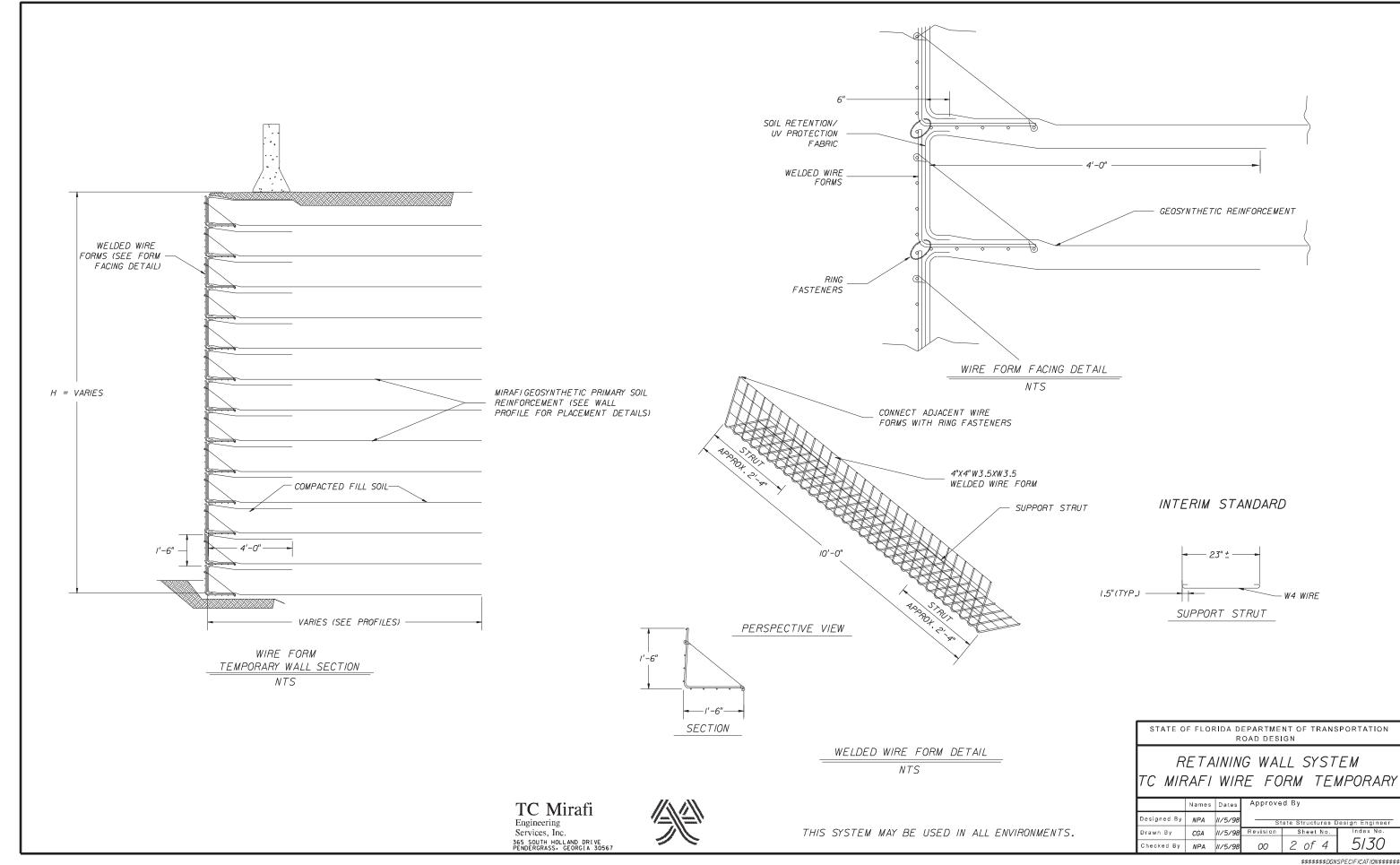
- 3.1 FOR LOCATION AND ALIGNMENT OF REINFORCED SOIL STRUCTURES, SEE RETAINING WALL CONTROL PLANS.
- 3.2 STEEL WIRE FORMS, REINFORCEMENT, SOIL RETENTION FABRIC, AND COMPACTED FILL SHALL BE PLACED IN SUCCESSIVE LIFTS IN THE SEQUENCE SHOWN IN THE CONSTRUCTION DRAWINGS.
- 3.3 GEOSYNTHETIC REINFORCEMENT SHALL BE PLACED AT THE ELEVATIONS. LOCATION, TYPE, ORIENTATION, AND TO THE LENGTHS SHOWN ON THE CONSTRUCTION DRAWINGS. THE REINFORCEMENT SHALL BE PLACED IN A MANNER SO AS TO AVOID SLACK OR WRINKLES. PINNING OR STAKES MAY BE REQUIRED TO MAINTAIN WRINKLE-FREE PLACEMENT DURING INSTALLATION.
- AT EACH REINFORCEMENT ELEVATION, BACKFILL SOILS SHALL BE COMPACTED TO A LEVEL SURFACE BEFORE PLACING THE REINFORCEMENT. ALL REINFORCEMENT SHALL BE PLACED NORMAL TO THE FACE OF THE WALL.
- 3.5 ADJACENT WIRE FORMS SHALL BE CONNECTED ALONG VERTICAL AND HORIZONTAL SEAMS WITH GALVANIZED INTERLOCKING FASTENERS PLACED 8 INCHES ON CENTER.
- 3.6 BACKFILL MATERIAL SHALL BE COMPACTED IN ACCORDANCE WITH FDOT SPECIFICATIONS - SECTION 548.
- TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE REINFORCEMENT. A MINIMUM FILL THICKNESS OF 6 INCHES IS REQUIRED FOR THE OPERATION OF TRACKED VEHICLES OVER THE REINFORCEMENT. TURNING OF TRACKED VEHICLES SHOULD BE AVOIDED TO PREVENT TRACKS FROM DISPLACING THE FILL AND THE REINFORCEMENT.
- RUBBER TIRED VEHICLES MAY PASS OVER THE REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE AVOIDED.
- TC MIRAFI ENGINEERING SERVICES, INC. IS RESPONSIBLE FOR THE INTERNAL STABILITY OF THE STRUCTURE ONLY, EXTERNAL STABILITY IS THE RESPONSBILITY OF OTHERS.

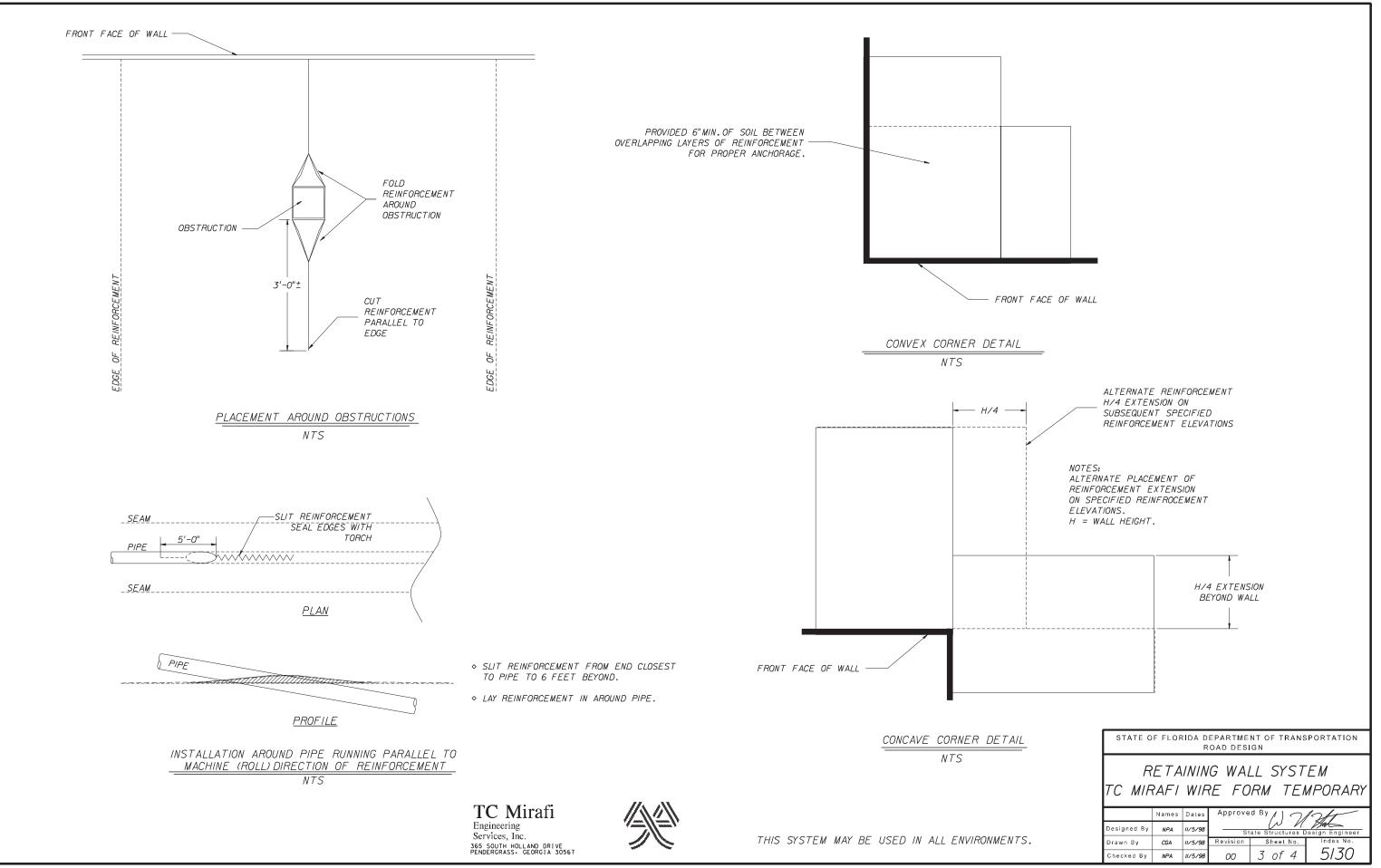
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RETAINING WALL SYSTEM TC MIRAFI WIRE FORM TEMPORARY

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awn By	CGA	11/5/98	Revision	Sheet No.	Index No.		
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THIS SYSTEM SHALL NOT BE USED FOR ACUTE ANGLE BIN WALLS. THIS SYSTEM MAY BE USED IN ALL ENVIRONMENTS.



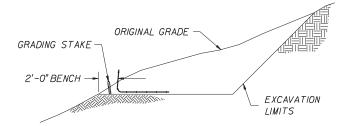


CONSTRUCTION SEQUENCE

- EXCAVATE FOR LEVEL BASE TO A LENGTH ADEQUATE FOR REINFORCEMENT EMBEDMENT.

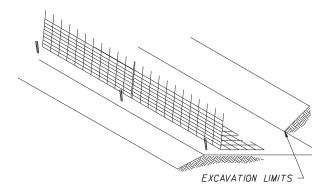
 SET GRADING STAKES AT A 6 INCHES OFFSET TO FACILITATE
- PROPER WIRE FORM ALIGNMENT.

 EMBED BOTTOM BASKET 6 INCHES BELOW FINISHED GRADE AT FRONT FACE OF WALL OR AS SHOWN ON WALL PROFILE.



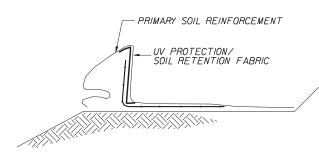
STEP 1

•FOR THE FIRST COURSE OF THE WALL, ALIGN BASKETS WITHOUT SPACES AND ATTACH WITH RING FASTENERS. •INSTALL STRUTS AT ABOUT 5 FOOT SPACING.



STEP 2

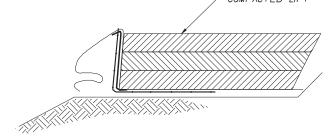
- •PLACE UV PROTECTION/SOIL RETENTION FABRIC AT ELEVATIONS AS SHOWN. •PLACE FACE FABRIC AGAINST WIRE FORM FACE.
- DRAPE FABRIC OVER WIRE FORM ALLOWING FOR THE REQUIRED WRAP EMBEDMENT.



STEP 3

- PLACE BACKFILL SOIL IN 6 INCHES MAXIMUM LIFTS. COMPACT SOILS WITHIN IM OF WIRE FORM USING LIGHT
- WEIGHT COMPACTION EQUIPMENT.

 COMPACT REMAINING BACKFILL SOILS WITH STANDARD COMPACTION EQUIPMENT TO REQUIRED DENSITY.

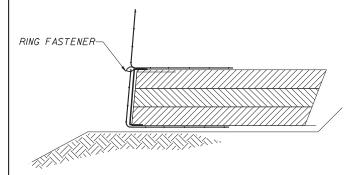


STEP 4

- PULL UV PROTECTION/SOIL RETENTION FABRIC AND PRIMARY REINFORCEMENT OVER COMPACTED FILL AND ANCHOR WITH SOIL.

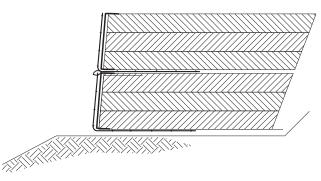
 • PLACE THE NEXT WIRE FORM AGAINST THE LOWER FORM AND ATTACH WITH RING FASTENERS.

 • INSTALL STRUTS ON SUCCEEDING LIFT.

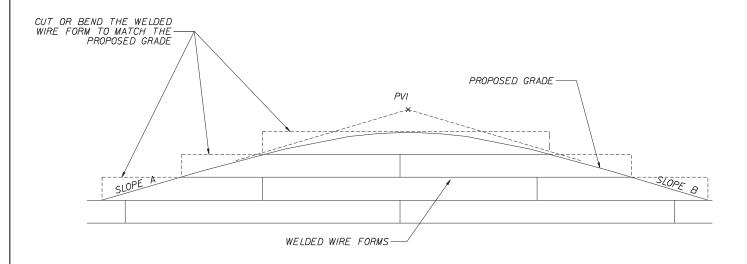


STEP 5

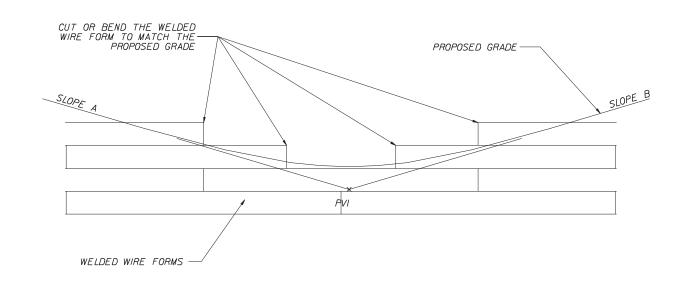
• REPEAT STEPS 2 THRU 5 UNTIL DESIRED HEIGHT OF WALL IS REACHED.



STEP 6



WELDED WIRE FORM ON VERTICAL CREST CURVE



WELDED WIRE FORM ON VERTICAL SAG CURVE

TC Mirafi Engineering Services, Inc.

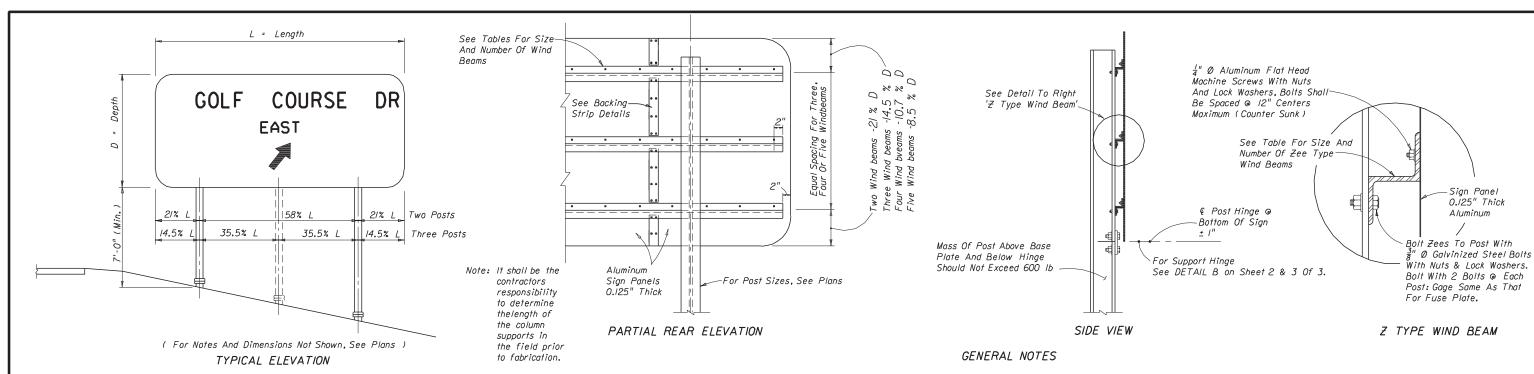
365 SOUTH HOLLAND DRIVE PENDERGRASS, GEORGIA 30567

RETAINING WALL SYSTEM TC MIRAFI WIRE FORM TEMPORARY

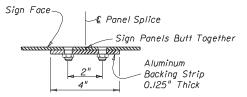
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

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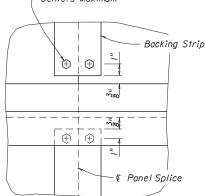
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Note: If the sign panels are deeper than 14', a Horizontal Panel Splice is allowed at an interior Z bar support, shop drawings shall be required. Minimum panel section width



Pairs Of 4 0 Aluminum Flat Head Machine Screws With Nuts And Lock Washers Spaced At I'-0" Centers Maximum



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BACKING STRIP DETAIL

DESIGN WIND SPEEDS BY COUNTY

ZONE NO.1 (60 mph)
Alachua, Baker, Bay, Bradford, Calhoun, Clay, Columbia, Escambia, Gadsden, Gilchrist, Hamilton, Holmes, Jackson, Jefferson, Lafayette, Lake, Leon, Liberty, Madison, Marion, Okaloosa, Putnam, Santa Rosa, Sumter, Suwannee, Union, Walton and Washington Counties.

ZONE NO. 2 (70 mph)

Citrus, Desoto, Dixie, Duval, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Levy, Nassau, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Seminole, St. Johns, Taylor and Wakulla Counties.

ZONE NO. 3 (80 mph))

Brevard, Charlotte, Collier, Indian River, Lee, Manatee, Martin, Palm Beach, Sarasota, St. Lucie and Volusia Counties.

ZONE NO. 4 (90 mph)
Broward. Dade and Monroe Counties.

	NUMBER OF WIND BEAMS FOR GIVEN DEPTH & WIND									
Wind	No. Beams	Max. Depth	Wind	No. Beams	Max. Depth					
60	2	8'-0"	80	2	6'-8"					
60	3	13'-4"	80	3	//' - 4"					
60	4	18'-0"	80	4	15' - 4"					
60	5	22'-8"	80	5	19'-0"					
70	2	7'-0"	90	2	6'-0"					
70	3	12'-0"	90	3	10'-4"					
70	4	16'-4"	90	4	14'-0"					
70	5	20'-8"	90	5	17'-8"					

SIZE OF WIND BEAMS					
	Length Of Sign (Feet)				
Size Of Zee*	2 Posts	3 Posts			
₹ 1.75 x 1.75 x 1.08	0 - //'-0"	0 - 17'-4"			
Z 3 x 2.69 x 2.33	11'-1"-19'-0"	17' -5" -29' -6"			
₹ 3 x 2.69 x 2.33	19'-1"- 20'-8"	29' -7" - 31' -6"			

*Note: Zees Are Aluminum - No Steel Equivalent Available Designation Gives (Member Depth) x (Width) x (Ib/ft) DESIGN SPECIFICATION Standard Specification for Structural Supports for Highway Signs, Luminaries and Traffic Signals, AASHTO 1994.

For welding refer to the latest editions of the AWS Structural Welding Codes for Steel and Aluminum, the AASHTO Standard Specifications for Welding

Structural Steel Highway Bridges, and the FDOT Standard Specifications with Supplement.

DESIGN WIND LOAD See Design Wind Speeds By County for wind in miles per hour on flat sign area. The allowable working stress shall be increased by 40% for combination dead load and wind load.

ALUMINUM MATERIALS All aluminum materials shall meet the requirements of the Aluminum Association's Alloy 6061-T6 and also the following ASTM specifications: Sheets and plates, B209; extruded tube, bars, rods & shapes, B221; and standard structural shapes, B308. Sheets are to be degreased, etched, neutralized and treated with Alodine 1200, Iridite 14-2, Bonderite 721, or equal. No stenciling permitted on sheets. Aluminum welding rods shall meet the requirements of Aluminum Association Alloy No. 5556 filler wire.

STRUCTURAL STEEL All structural steel shall meet the requirements of ASTM A709 Grade 36.

ALUMINUM BOLTS, NUTS, Aluminum bolts shall meet the requirements of Aluminum Association Alloy 2024-T4 (ASTM F468). The bolts shall have an anodic coating at least 0.0002"

& LOCKWASHERS thick and be Chromate sealed. Lock washers shall meet the requirements of Aluminum Association Alloy 7075-T6 (ASTM B221). Nuts shall meet the requirements of Aluminum Association Alloy 6061-T6 or 6262-T9 (ASTM F467).

STEEL BOLTS, NUTS, All steel bolts, nuts and washers shall meet the requirements of ASTM A325 Types I & 2 and shall be galvanized in accordance with Standard Specifications 962-7. & WASHERS

ALTERNATE MATERIAL Material meeting the requirements of ASTM B209 or Aluminum Association Alloys 5/54-H38 or 5052-H38 may be used for sheet and plate. Material meeting the requirements of Aluminum Association Alloy 635/-T5 and ASTM B22/ may be used for extruded bars, rods, shapes and tubes.

TOLERANCES All above materials shall be in accordance with the governing ASTM specifications.

GALVANIZING All steel shapes, angles, tees, plates, bolts, nuts and washers shall be galvanized in accordance with Standard Specifications 962-7.

BASE CONNECTION High strength bolts L2 in the base connection shall be tightened only to the torque shown in the tables on sheets 2 & 3 of 3. Overtightened base connections will not be accepted.

FUSE PLATES All holes in fuse plates shall be drilled. All plate cuts shall, preferably, be saw cuts; however, flame cutting will be permitted provided all edges are ground. Metal projecting beyond the plane of the plate face will not be tolerated.

SIGN FACE All sign face corners shall be rounded. See Sign Layout Sheet.

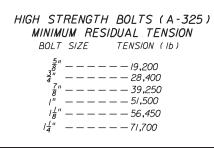
SHOP DRAWINGS When ground sign supports are fabricated in accordance with these plans no shop drawings are required. Shop drawings will be required for approval when the column length exceeds the length shown in the plans by more than 2'-0". However, shop drawings for sign panels, messages, lettering and quantities shall be submitted to traffic plans for approval.

FABRICATOR NOTE All bolted connections, except L₂ bolts and Zee to Post bolts, shall be high strength bolts. Bolts shall be tightened in the shop following a method approved by the engineer. Tightening shall be to such a degree so as to attain in each bolt the residual tension specified in the tabulation below:

FOUNDATION Contractor may use precast foundations in pre-drilled holes a minimum of 12" larger than the foundation indicated on the plans in either wet or dry conditions.

The holes shall be clean and without loose material. Temporary casing shall be required if the soil is unstable. The holes shall be filled with flowable concrete after the precast foundation is in place. The cost of flowable concrete, installing and removal of casing shall be included in the unit price of

Sign Multi-Post.

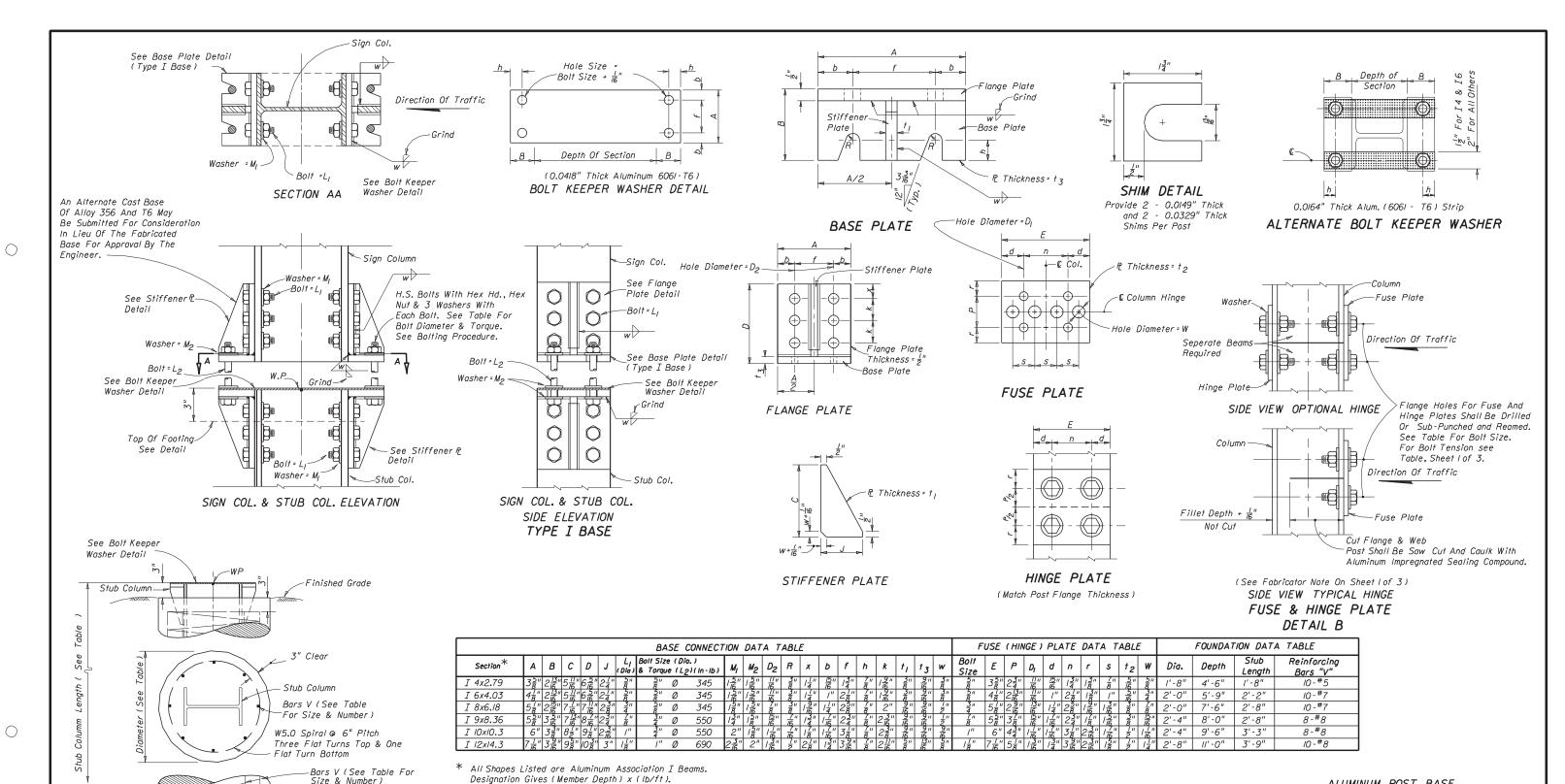


SIGN PANEL AND WIND BEAMS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

STANDARD ROADSIDE SIGN BREAK-AWAY PANEL DETAIL

	Names	Dates	Approve	$^{\circ}$ By $//$	12/	
Designed By	RES	11-94	State Structures Design Engineer			
Drawn By	DDDS	11-94	Revision	Sheet No.	Index No.	
Checked By	DER	11-94	00	1 of 3	9535	



PROCEDURE FOR ASSEMBLY OF BASE CONNECTION: FOR BOLTS L2

- I. Assemble post to stub with bolts and with one flat washer on each bolt between plates.
- 2. Shim as required to plumb post (See Shim Detail).
- 3. Tighten all bolts the maximum possible with I'-0" to I'-3" wrench to bed washers and shims and to clean bolt threads then loosen each bolt in turn and retighten in a Systematic order to the prescribed torque (See Table).
- 4. Burr threads at junction with nut using a center punch to prevent nut loosening.

Sections shown are for installation on right shoulder and in gore. Plate slot bevels are opposite hand from that shown for installations in the median.

ALUMINUM POST, BASE, FOUNDATION & FUSE & DETAILS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

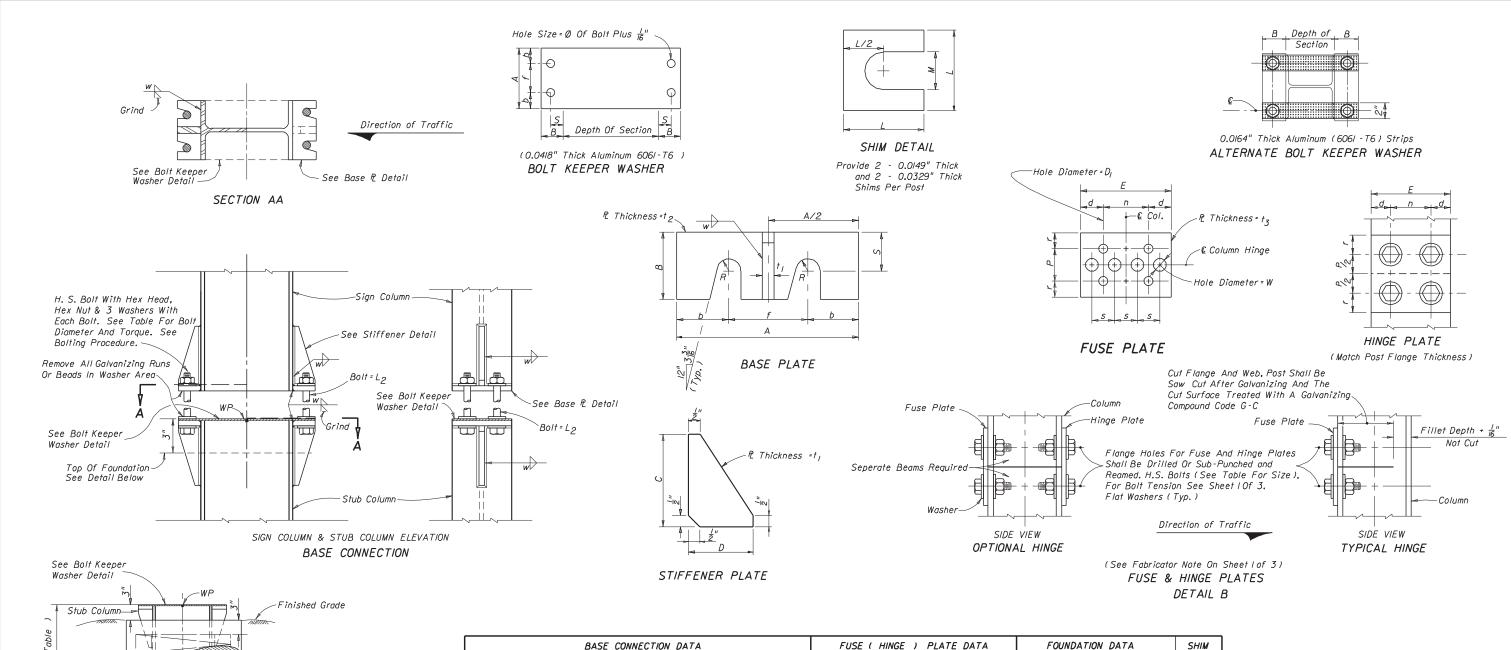
STANDARD ROADSIDE SIGN BREAK-AWAY POST DETAILS

	Names	Dates	Approve	d By /)	12/	
Designed By	RES	11-94	State Structures Design Engineer			
Drawn By	n By SGF 11-94 Revision		Sheet No.	Index No.		
Checked By	DER	11-94	00	2 of 3	9535	

\$\$\$\$\$\$\$SYT/MF\$\$\$\$\$

Class II Concrete FOUNDATION DETAIL NOTES: To prevent galvanic corrosion, reinforcing steel shall not be in contact with the aluminum stud column. All reinforcing to be Grade 60.

W5.0 Spiral



Stub Cole	WP Finished Grade
ength (Stub Column W5.0 Spiral @ 6" Pitch; Three Flat Turns Top & One Flat Turn Bottom Bars V (See Table For Size & Number)
=	Bars V (See Table For Size & Number) W5.0 Spiral

Class II Concrete

 \bigcirc

PROCEDURE	FOR	ASSEMBLY	0F	BASE	CONNECTION

I. Assemble post to stub with bolts and with one flat washer on each end bolt between plates.

B C D Boll Size(L2) R

" 2" $5\frac{1}{8}$ " 2" $\frac{5}{8}$ " \emptyset 345 " $2\frac{3}{6}$ " $6\frac{1}{4}$ " $2\frac{1}{4}$ " $\frac{3}{4}$ " \emptyset 550

 $2\frac{3}{4}$ 8" $2\frac{3}{4}$ $I_{\overline{R}}$ 0 780

 $6\frac{1}{8}$ " $2\frac{3}{8}$ " 8" $2\frac{3}{8}$ " $\frac{7}{8}$ " \emptyset 640

8" 3" 8" 3" In 0 780

* Designations Give (Nominal Depth) x (Ib/ft)

&Torque(in -Ib

2. Shim as required to plumb post (see shim detail).

Section*

W 6x12

W 8x/8

W 10x22

W 10x33

W 12x40

- 3. Tighten all bolts the maximum possible with I'-O" to I'-3" wrench to bed washers and shims and to clean bolt threads then loosen each bolt in turn and retighten in a systematic order to the prescribed torque (see table).
- 4. Burr threads at junction with nut using a center punch to prevent nut loosening.

NOTE:

 $|D_i|d|n|r$

Sections shown are for installation on right shoulder and in gore. Plate slot bevels are opposite hand from that shown for installations in the median.

Dia.

2'-0"

2'-4"

Depth

7'-6"

8'-6"

10'-3"

Length Bars V

2'-4" 10-#7

2'-10" 10-#7

3'-4" 8-#8

4'-0"

8-#8

2" 15"

STEEL POST, BASE, FOUNDATION & FUSE & DETAILS

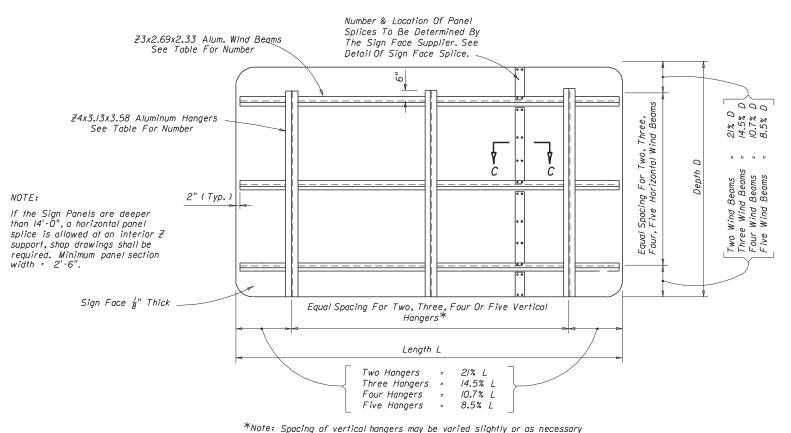
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

STANDARD ROADSIDE SIGN BREAK-AWAY POST DETAILS

	Names	Dates	Approved By				
esigned By	RES	11-94	State Structures Design Engineer				
rawn By	SGF	11-94	Revision	Sheet No.	Index No.		
Checked By	DER	11-94	00	3 of 3	9535		

NOTE; All Reinforcing To Be Grade 60.

FOUNDATION DETAIL



(1) For "General Notes" covering Material Specifications; see Sheets 10f 3, Index 9535.

(2) This dimension shall be adjusted for porcelain enameled sign panel.

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Number Of Z3x2.69x2.33 Horiz. Wind Beams For Sign Depth And Wind			Number Of Z4x3.l3x3.58 Vertical Hanger Beams For Sign Length					
Wind			2 Hangers	3 Hangers	4 Hangers	5 Hangers		
M.P.H.	No. Beams	Max. Depth	Sign Length	Sign Length	Sign Length	Sign Length		
110	2	5'-0"	0-15'-0"	/5'-/"-30'-0"	30'-1"- 45'-0"			
110	3	8'-6"	0-15'-0"	15'-1"-30'-0"	30'-1"- 45'-0"			
110	4	//'-6"	0-/3'-0"	13'-1"-18'-3"	18'-4"- 24'-9"	24'-10"- 31'-4"		
110	5	14'-0"	0-/3'-0"	13'-1"-18'-3"	18'-4"- 24'-9"	24'-10"- 31'-4"		
100	2	5'-3"	0-15'-0"	15'-1"-30'-0"	30'-1"- 45'-0"			
100	3	8'-10"	0-15'-0"	15'-1"-22'-3"	22'-4"- 30'-0"	30'-1"- 38'-0"		
100	4	12'-0"	0-15'-0"	15'-1"-22'-3"	22'-4"- 30'-0"	30'-1"- 38'-0"		
100	5	15'-0"	0-11'-7"	11'-8"-16'-4"	16'-5"- 22'-2"	22'-3"- 28'-0"		
90	2	5'-6"	0-15'-0"	15'-1"-30'-0"	30'-1"- 45'-0"			
90	3	9'-6"	0-15'-0"	15'-1"-27'-3"	27'-4"- 37'-0"			
90	4	12'-9"	0-15'-0"	15'-1"-27'-3"	27'-4"- 37'-0"			
90	5	16' -0"	0-14'-3"	14' - 4" - 20' - 0"	20'-1"- 27'-0"	27'-1"- 34'-3"		
80	2	6'-0"	0-15'-0"	15'-1"-30'-0"	30'-1"- 45'-0"			
80	3	10'-0"	0-15'-0"	15'-1"-30"-0"	30'-1"- 45'-0"			
80	4	14'-0"	0-15'-0"	15'-1"-25'-9"	25'-10"- 34'-10"			

*Note: Spacing of vertical hangers may be varied slightly or as necessary to clear the truss struts and diagonals at panel points. TYPICAL SIGN FACE ELEVATION FOR OVERHEAD TRUSS See Note 2 -See Note 2 Varies, 6' maximum Varies, 6' Cantilever except where maximum Cantilever Sign Depth is 20' or -6"± 6"± é Panel Splice except where Sign greater; then 7' Sign Face cantilever is maximum Depth is 20' or Sign Panels Butt Together greater; then 7' cantilever is maximum Structural В Tee Chord Backing Strip **†** B ∦" Thick SECTION C-C Sign Face #" Thick Sign Face Std. Gage ∠ Aluminum 5" x 3½" x 3/8" Panel Spìice Z4x3./3x3.58 Hanger (±) | (±) \bigoplus See Detail A See Detail A-5⊓ Ø Hex. Hd. Bolts, §" Ø Hex Hd. Bolts, Alum. Or Galv. Steel Alum. Or Galv. Steel ⊕ ¦ \oplus W/Hex Nuts And Lock W/Hex Nuts And Washer (2 Reg'd.) Pairs Of $\frac{1}{4}$ " Ø Aluminum Lock Washers Flat Head Machine _€ Bottom Chord SECTION B-B Screws Spaced At 12" Centers Maximum -D/2 + 1= " ± Bottom Chord BACKING STRIP DETAIL GENERAL NOTES FOR ROUND CHORD MEMBERS FOR ROLLED STRUCTURAL SHAPES

(LIGHTING NOT SHOWN)

TYPICAL DETAILS OF SIGN & TRUSS CONNECTION

Bolt Wind Beam To Vertical
Hanger With 5" Ø Aluminum
Hex Head Bolt With Nut & Z4x3./3x3.58
Lock Washer

(SHOWING ATTACHMENT OF SIGN FACE
PANEL TO VERTICAL HANGER SUPPORTS)

Sign Face $\frac{1}{8}$ " Thick

DETAILS OF SIGN FACE & TRUSS CONNECTION

DETAIL A

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

½" Ø Alum. Flat Head Machine Screws With Nuts And Lock

Z3x2.69x2.33

Aluminum

Wind Beam

Washers. Screws Shall Be Spaced at 12" Centers

Maximum

ALUMINUM & STEEL OVERHEAD SIGN STRUCTURES

Designed By RES 11/94 Approved By State Structures Design Engineer

Drawn By DDDS 11/94 Revision Sheet No. Index No.

Checked By DER 11/94 00 10f 1 11/037

\$\$\$\$\$\$\$SYTIME\$\$\$\$\$

an Sation Ser		SIGN		TYP	YPE OF SIGN BRACKET			sat to a			TYPE OF SIGN BRACKET			uoi poj Lega SIGN			TYPE OF SIGN BRACKET						
Sign Identfication Number	PROFILE	- SIZE	SQ. FT.			ZONE	1 00	Sign Identfication Number	PROFILE	- SIZE	SQ. FT.		WIND 70	ZONE		Sign Identfication Number	PROFILE	- SIZE	SQ. FT.	60	WIND 70	ZONE 80	90
1	∇	24 x 24	1.7	60 2 - I	70 2 - I	2 - I	90 2 - I	30	日	15 x 30	8.1	1 - I	/ - I	/ - I	90 / - I	55		30 x 24	5.0	2- I	2- I	2- I	2- I
2	∇	30 x 30	2.7	2 - I	2 - I	2 - I	2 - I			24 x 30 15 x 21		2- I I- I	2- I - I	2- I 1- I	2- I 1- I	56	\triangleright	36 x 48	5.6	2- Ⅱ	2- II	2- II	2- Ⅱ
3	∇	36 x 36	3.9	2 - I	2 - I	2 - I	2 - I	31		36 x 30	9.7	2- I	2- I	2- I	2- I	57		24 x 36	6.0	2- I	2- I	2- I	2- I
4	∇	48 x 48	6.9	/ -II & / -I	/-II & /-I	/-II & /-I	l -II &	32		15 x 30 36 x 30	10.6	1 - I 2- I	1 - I 2 - I	/ - I 2- I	1 - I 2- I	58		36 x 24	6.0	2- I	2- I	2- I	2- I
5	∇	60 x 60	10.8	DO NO	T USE	SINGLE	COLUMN			12 x 24 24 x 24	8.2	1 - I 2 - I	1 - I 2- I		1 - I 2 - I	59		30 x 30	6.3	2- I	2- I	2- I	2- I
6	0	36 Ø	7,1	2 - I	2 - I	2 - I	2 - I	33		15 x 21		/ - I	1 - I	/ - I	/ - I	60	\Diamond	30 x 30	6.3	2- I	2- I	2- I	3- I
7	0	48 Ø	12.6	2 - II	2 - 11	2 - 11	2 - 11	7.4		15 x 30 24 x 24	9.3	1 - I 2 - I	1 - I 2- I	1 - I 2 - I	/ - I 2- I	61		36 x 36	6.75	2- I	2- I	2- I	2- I
8	\bigcirc	18 x 18	1.9	2 - I		2 - I		34		15 x 21		/ - I	1 - I	/ - I	/ - I	62		30 x 36	7.5	2- I	2- I	2- I	2- I
9	\bigcirc	24 x 24	3.3	2 - I		2 - I		35		12 x 24 24 x 30	9.2	1 - I 2- I	1 - I 2- I	1 - I 2- I	1 - I 2- I	63		36 x 30 24 x 48	7.5 8.0	2- I 2- II	2- I 2- II	2- I 2- II	2- II
10	0	30 x 30	5.2	2 - I		2 - I				15 x 21		/ - I	/ - I	/ - I	/ - I	65		12 x 36	8.2	1 - I	/ - I	/ - I	1 - I
12	0	36 x 36 48 x 48	7.5 	2 - I 2 - II		2 - I 2 - π	2 - I 2 - II	36		15 x 30 24 x 30	10.3		1 - I 2 - I 1 - I			66		30 x 30 30 x 42	8.8	2- I 2- I	2- I 2- I	2- I 2- II	2- I
\vdash		12 x 24	15.5	1 - I	1 - I	1 - I	/ - I			15 x 21		/ - I	/ - I	/ - I	/ - I	67		36 x 36	9.0	2- I	2- I	2- I	2- I
/3		24 x 24	5.4	2- I	2- I	2- I	2- I	37		12 x 24 , 12 x 24 24 x 24 , 24 x 24 15 x 21	13.6	2- II 1- I	2- II 1- I	2- II 1- I	2- II 1- I	68	\Diamond	36 x 36	9.0	2- I & /- II	2- I 8 1- II	2- I 8 1- II	2- I & II
14		15 x 30 24 x 24	6.5	1 - I 2- I	/ - I 2- I	1 - I 2 - I	1 - I 2- I			12 x 24 , 12 x 24		/ - II	/ - II	/ - II	/ - II	69	8	12 x 36 30 x 30	9.3	1 - I 2 - I	/ - I 2 - I	1 - I 2 - I	1 - I 2 - I
15	8	12 x 24	6.3	1 - I	/ - I	/ - I	/ - I	38		24 x 24 , 24 x 24 15 x 21 , 15 x 21	15.2	2- II 1- II	2- II 1- II	2- II 1- II	2- II 1- II	70	\triangle	30 x 30 18 x 24	9.3	2- I 2- I	2- I 2- I	2- I 2- I	3- I 2- I
\vdash		24 x 30 15 x30		2- I I- I	2- I - I	2- I	2- I 1- I			12 x 24 , 12 x 24 24 x 24 , 24 x 24	15.4	/ - <u>П</u>	/ - II	1 - II 2- II	/ - II	71	\triangleright	48 x 64	9.9	DO NO	T USE SI	NGLE CO	LUMN
16	\bigcirc	24 x 30	7.4	2- I	2- I	2- I	2- I	39		15 x 21 , 15 x 21	16.4	2- II I- II	2- II 1- II	2- II - II	2- II - II	72		30 x 48	10.0	2- 11	2- II	2- II	2- Ⅱ
17		15 x 30 36 x 36	10.8	1 - I 2- I	/ - I 2- I	1 - I 2 - I	1 - I 2- I			15 x 30 , 15 x 30 24 x 30 ,24 x 30	19,2	מ אמ	 T USE S	 SINGLE L	 משוווסי	73	5	12 x 36 36 x 36	10.5	1 - I 2- I	1 - I 2- I	1 - I 2- I	1 - I 2 - I
18	Ě	15 x 30	12.6	1 - I	/ - I	/ - I	/ - I	40	<u> </u>	15 x 21 , 15 x 21		20 110				74		30 x 54	//.3		T USE S		
	\square	36 x 45	12.0	2- I	2- Ⅱ	2- Ⅱ	2- Ⅱ			12 x 24 , 12 x 24 12 x 24 , 12 x 24		/ - П / - П		/ - II	/ - II	75		36 x 48	12.0	2- II	2- II	2- II	2- II
19	\Box	15 x 30 48 x 48	16.7	1 - I 2- II	/ - I 2- Ⅱ	1 - I 2 - II	1 - I 2- II	41		24 x 24 , 24 x 24 15 x 21 , 15 x 21	20.4	2- II - II	2- II - II	2- II - II	2- II - II	76	$\stackrel{\square}{\geq}$	48 x 36 36 x 36	12.0	2- I 2-I& I-II	2- I 2-I& /-II	2- I 2-I& I-II	2- I 2-I& -II
20	\(\text{\tin}\text{\ti}\}\text{\text{\text{\text{\text{\text{\text{\text{\tex{\tex	15 x 30	20.1	DO NO	T USE .	SINGLE	COLUMN			15 x 21			1 - I			78		18 x 24 48 x 48	12.0	2- I - I &	2- I /- I &	2- I /- I &	2- I - I &
		48 x 60 12 x 24		1 - I	/ - I	/ - I	/ - I	42		24 x 24 12 x 24 .12 x 24	22.6	2- I 1- II	2- I I- II	2- I 1- II		79		30 x 60	12.5		Ţ [®] Π T USE S		1 1
21		24 x 24 15 x 21	7.6	2- I I- I	2- I	2- I - I	2- I			24 x 24 , 24 x 24 15 x 21 , 15 x 21		2- II 1- II	2- II - II	2- Ⅱ	2- Ⅱ	80		48 x 48	16.0	2- Ⅲ	2- II		2- II
22	层	15 x 30	0.7	1 - I	/ - I	1 - I	/ - I			12 x 24 24 x 30		, 11	, <u> </u>	, <u>n</u>	, 11	81	\Diamond	48 x 48	16.0	2- I & /- II	2- I 8 I- II	2- I & I	2- I & II
		24 x 24 15 x 21	8.7	2- I I- I	2- I - I	2- I - I		43		15 x 21 12 x 24 , 12 x 24	25.6	DO NO	I T USE S	I SINGLE (COLUMN	82		30 x 78	16.3		T USE S		
23		12 x 24 24 x 30	8. 5	1 - I 2- I	1 - I 2 - I	1 - I 2 - I				24 x 24 , 24 x 24 15 x 21 , 15 x 21						83		30 x 84	17.5	DO NO	T USE S	INGLE CO	DLUMN
	Ě	15 x 21 15 x 30		1 - I		/ - I		44		18 x 12	1.5	/ - I	/ - I	/ - I	/ - I	84		48 x 54	18.0	DO NO	T USE S	INGLE CO	DLUMN
24		24 x 30 15 x 21	9.6	2- I I- I	2- I	2- I 1- I	2- I	45		12 x 36	3.0	/- I	1- I	1- I	/- I	85		42 x 66	19.3	DO NO	T USE S	INGLE CO	DLUMN
		12 x 24	6.0	1 - I	/ - I	/ - I	/ - I	46		18 x 24	3.0	2- I	2- I	2- I	2- I	86		60 x 48	20.0	3- Ⅲ	3- Ⅱ	3- Ⅱ	3- Д
25		24 x 24	6.0	2- I	2- I	2- I	2- I	47		24 x I8	3.0	2- I 2- I	2- I 2- I	2- I 2- I	2- I 2- I	87		66 x 48	22.0	3- Ⅲ	3- II	3- II	3- Д
26		24 x 24 15 x 21	6.2	2- I I- I	2- I I- I	2- I I- I	2- I I- I	48		18 x 18 9 x 12	3.0	1 - I	1 - I	1 - I	/ - I	88		60 x 72	30.0	DO NO	T USE S	INGLE CO	DLUMN
		15 x 30	7 '	1 - I	/ - I	/ - I	/ - I	49		18 x 30	3.8	2- I	2- I	2- I		89		96 x 48	32.0		T USE S		-
27		24 x 24	7.1	2- I	2- I	2- I	2- I	50		30 x 40	3.9	2- I	2- I	2- I		90		24 x 78	/3.0		T USE S		
28		12 x 24 24 x 30	7.0	1 - I 2- I	/ - I 2 - I	/ - I 2- I	1 - I 2- I	5 <i>l</i> 52	\Diamond	24 x 24 24 x 24	4.0	2- I	2- I	2- I 2- I	2- I 2- I	91		36 x 78	/9.5	DO NO	T USE S	INGLE C	DLUMN
20		24 x 30	7.0	2- I	2- I	2- I	2- I	53		18 x 36	4.0	2- I 2- I	2- I 2- I	2- I	2- I								
29		15 x 21	7.2	1 - I	1 - I	/ - I	/ - I	54		30 x 30	4.7	2- I	2- I	2- I									
														ļ									

NOTE:

The Gore Exit Panel (FTP-31, Index 17355, Sheet 3), Sign identification Number 88, can be installed on a single column with the following stipulations:

- I. Maximum height to bottom of sign is I4'. 2. Column size is 6" aluminum round tube with $\frac{1}{4}$ " wall.
- 3. 3 Type II Brackets required for attachment.
 4. For Type II Bracket details, Attachment and General Notes see Index No. II860.
 5. For Footing size and Slip Base Details, see Index No. II863.

Sign size is in Inches unless other wise specified.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SINGLE COLUMN GROUND SIGNS

	Names	Dates	Approve	d By /) (12/			
Designed By	RES	10/34	State Structures Design Engineer					
Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.			
Checked By	DER	11/94	00	I of 3	11860			

GENERAL NOTES

GENERAL SPECIFICATIONS: Florida Department of Transportation Standard Specifications for Road and Bridge Construction (1999) and Supplements thereto.

DESIGN SPECIFICATIONS: Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals, AASHTO 1994.

ALUMINUM : Except as noted below, Aluminum Materials shall meet the requirements of Aluminum Association Alloy 6061-T6 (ASTM B209 , B221, or B308).

I. Permitted Alternate for Sheets and Plates --- Alloy 5/54-H38 (ASTM-B209)

CONCRETE: All concrete shall be Class I (Special), the specified compressive strength at 28 days (f'c) shall be 3 ksi min.

SIGN PANELS: Sign Panels shall be $\frac{1}{8}$ " min. thick Aluminum Plate with all corners rounded. See sign layout sheet. Panels are to be degreased, etched, neutralized and treated with Alodine 1200, Irdine 14-2, Bonderite 721 or equal. No stenciling permitted on panels.

ALUMINUM BOLTS, NUTS & LOCKWASHERS: Aluminum bolts shall meet the requirements of ASTM F468, Alloy 2024-T4.

The Bolts shall have an Andoic Coating of at least 0.0002 inches thick and be chromate sealed. Lockwashers shall meet the requirements of Aluminum Association Alloy 7075-76 (ASTM B221). Nuts shall meet the requirements of ASTM F-467, Alloy 6061-76 or 6262-79.

STAINLESS STEEL BOLTS, NUTS AND LOCKWASHERS: Stainless Steel Bolts, Nuts and Lockwashers conforming to ASTM F593 Alloy Group 2 Condition A, CW2, or SH4 may be provided in lieu of Aluminum Bolts, Nuts and Washers.

U-BOLTS, NUTS & LOCKWASHERS: U-Bolts, Nuts and Lockwashers shall meet the requirements of ASTM A307, Grade A and shall be galvanized in accordance with ASTM A153.

INSTALLING FRANGIBLE COLUMN SUPPORTS: Columns (Posts) may be installed by driving the columns in accordance with index Nos. II861 thru II865, or as an alternate method the contractor may set the columns (Posts) to the depth indicated in preformed holes backfilled with suitable material tamped in layers not thicker than 6" to provide adequate compoction.

SHOP DRAWINGS: When Type C ground sign supports are furnished and fabricated in accordance with these plans, shop drawings will NOT be required for approval by the Engineer.

HOW TO USE THIS TABLE: Select the appropriate Sign Profile and Size to determine the Sign Identification Number. If the exact Sign Size of all Components are not listed, select the appropriate profile and larger Component Sizes. This table also gives the Quantity and Type of Sign Brackets required for each Sign for each Wind Zone. Where the Sign Size is given as a Vertical and Horizontal Dimension, the Vertical Dimension (Depth) is given first and the Horizontal Dimension (Length) is given last. For Column Sizes, Heights and Footings see appropriate (Wind Zone or Height = I4' Max.) sheets titled "Column Sizes, Column Heights and Footings" Index Numbers 11861 thru 11865. No Shop or Field Splice is allowed in Sign Panels. All Panels shall be furnished in one piece.

WIND SPEEDS BY COUNTY

ZONE NO. 1(60 M.P.H.)

Alachua, Baker, Bay, Bradford, Calhoun, Clay, Columbia, Escambia, Gadsden, Gilchrist, Hamilton, Holmes, Jackson, Jefferson, Lafayette, Lake, Leon, Liberty, Madison, Marion, Okaloosa, Putnam, Santa Rosa, Sumter, Suwannee, Union, Walton and Washington Counties.

ZONE NO. 2 (70 M.P.H.)

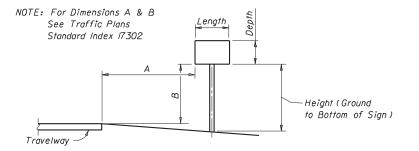
Citrus, De Soto, Dixie, Duval, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Levy, Nassau, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Seminole, St. Johns, Taylor and Wakulla Counties.

ZONE NO. 3 (80 M.P.H.)

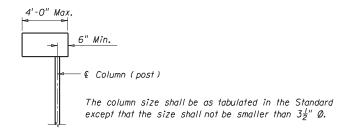
Brevard, Charlotte, Collier, Indian River, Lee, Manatee, Martin, Palm Beach, Sarasota, St. Lucie and Volusia Counties.

ZONE NO. 4 (90 M.P.H.)

Broward, Dade and Monroe Counties.



TYPICAL SECTION



Note: All cantilever sign installations shall comply with standard Index 17302.

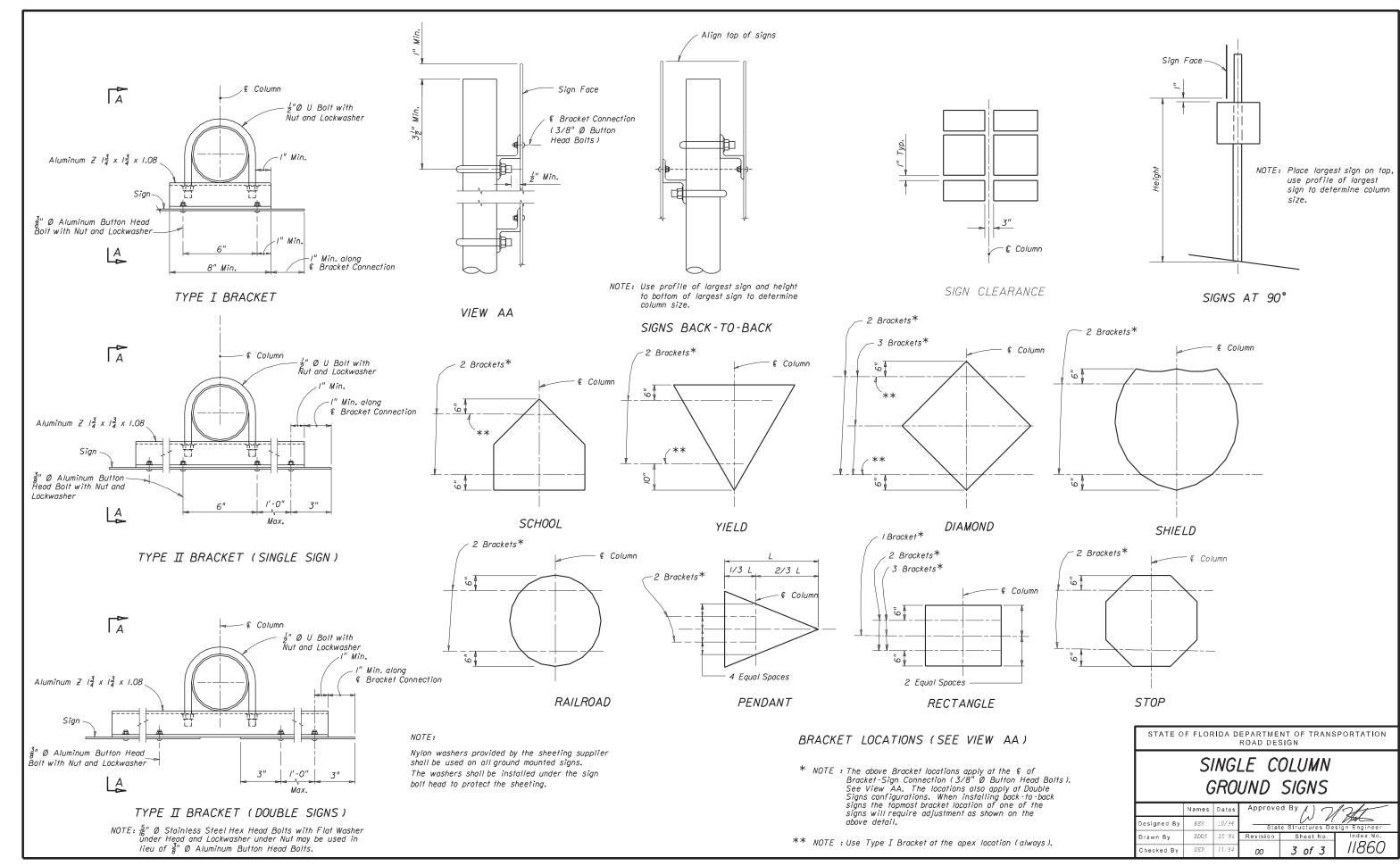
The sign shall be supported by an aluminum round column with concrete footing and breakaway support. All sign brackets shall be Type II.

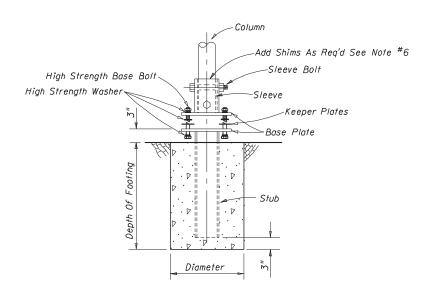
CANTILEVER SIGN

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

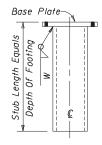
SINGLE COLUMN GROUND SIGNS

	Names	Dates	Approved By							
Designed By	RES	10/34	State Structures Design Regineer							
Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.					
Checked By	DER	11/94	00	2 of 3	11860					

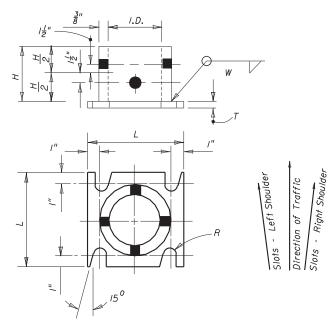




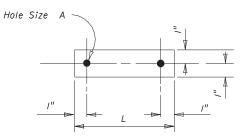
SLIP BASE AND FOOTING DETAIL



Stub Size Equals Min. Sleeve Size Or Longer
STUB DETAIL



SLEEVE & BASE PLATE DETAILS



0.04" Thick Alum. Strip-2 Req'd Per Base BOLT KEEPER DETAIL

SLIP BASE DETAILS

Note: Unless noted otherwise, all dimensions are in inches

Column	Sleeve	Sleeve	Weld	Base	Plate	Radius	Base	Bolt	Base Bo	It Torque	Hole
Size	I.D. (Max)	Height H	W	L	T	R	Size	Length	Ft-lbs	In-Ibs	Size A
4 x ½	4 <u>/</u>	6	<u>5</u>	8	<u>3</u>	<u> </u> 32	<u>5</u>	3	29	355	<u>11</u> 16
$4\frac{1}{2} \times \frac{1}{4}$	4 <u>9</u>	6	<u>5</u> 8	8	7 8	<u>//</u> 32	<u>5</u>	3 ½	29	<i>3</i> 55	<u>//</u>
5 x ½	5 <u>/</u> 6	7	5 <u>8</u>	8	7 8	<u> </u> 32	<u>5</u>	3½	29	355	<u> </u> 6
6 x ½	6 <u>/</u>	8	<u>//</u>	9	/	<u>7</u> 16	<u>3</u>	3 <u>/</u>	48	580	<u>13</u> 16
8 x 5/6	8 <u>/</u>	10	<u>3</u>	//	/	1/2	7 8	3 <u>3</u>	53	640	<u>15</u> 16

NOTES

- I. Work this Standard with Standard Index Numbers 11860 and 11865.
- To determine column (post) size and footing requirements use the required Sign Identification Number and Sign Height (H), Designs for Heights (H) lower than those listed in the Table are included in Standard Index Number II865.
- 3. Single Column installations are not allowed for heights (H) exceeding the maximum height shown in the Table, and for sign profiles (Sign Identification Numbers) without any design tablulated. In this event, the sign(s) will have to be supported by multiple columns (posts) featuring breakaway devices. See Standard Index Number 9535.
- 4. The Column (Post) material shall be aluminum. The size is given as outside diameter and wall thickness. Columns (posts) larger than $3\frac{1}{2}$ " x $\frac{1}{6}$ " are non-fragible and shall be installed with breakaway supports and will have concrete footings and slip bases.
- 5. The foundation size is given as outside diameter and depth.
 - a) Frangible Supports: Foundations for Frangible Supports do not require concrete.

 The column (post) shall be driven into the ground to the depth indicated.
 - b) Breakaway Supports: Foundations for Breakaway Supports require concrete. The column support shall be set in a concrete foundation, sized as shown in the table. The first dimension indicates the diameter and the second dimension the depth into the ground. In all cases the ground is to be considered as undisturbed earth, road material, or properly compacted fill.
- 6. SLIP BASE NOTES :
 - a) The Inside Diameter (I.D.) of the sleeve shall be no more than $\frac{L''}{16}$ larger than the Outside Diameter (O.D.) of the Column.
 - b) The sleeve bolts shall be $\frac{1}{2}$ $\!\!\!/\!\!\!/\!\!\!/$ $\!\!\!/\!\!\!/$ with locknuts. The bolts shall be galvanized steel (ASTM A-307) or Aluminum Association Alloy 2024-T4 or 6061-T6 (ASTM R-2II).
 - c) The base bolts, nuts and washers shall be high strength ASTM A-325 and shall have an electroplated zinc coating SC3, Type II applied in accordance with ASTM B633.
 - d) An alternate cast base of aluminum alloy 356 and T6 temper in lieu of the fabricated base may be submitted for approval by the Engineer. If a cast base is used the stub will be the same as the column and will be bolted to the casting.
 - e) Assemble the slip base connection in the following manner:
 Cannect column to sleeve using two (2) ½" Ø machine bolts.
 Assemble top base plate to stub base plate using high strength bolts with three (3) hardened washers per bolt. One (1) washer per bolt and two (2) bolt keeper plates go between the base plates.
 Use shim stock as required to plumb the column.
 Tighten all bolts the maximum possible with a 12" to 15" wrench to bed the washers and shims and to clear the bolt threads. Loosen each bolt one (1) turn and retighten to the prescribed torque (see table). Bolts shall be tightened with properly calibrated wrenches under the supervision
 - of the project engineer. Burr threads at junction with nut using a center punch to prevent nut loosening. f) Use galvanized steel shims to obtain a tight fit between the column face and the sleeve. Place shims in all quadrants between the $\frac{1}{2}$ " Ø sleeve bolts.

The shim length shall be I" shorter than the height of the sleeve.

COLUMN SIZE, COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SINGLE COLUMN GROUND SIGNS

60 W.P.H.
WIND
LOADING

Designed By DER 10/34 Approved By Designed By Designed By State Structures Design Engineer Revision Sheet No. Index No. 1062

COL. SIZE																	
FOUNDATION	0 x 4-0	0 x	4-3	0 x	4-9	0 x	5-3	2-0	x 3-9	2-0	x 4-0	2-0	x 4-3	2-0	x 4-9	2-0 x	4-9
Sign Identification							HEIGH	IT (F	eet)								
Number	(+) to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to
1	15	15	20	20	25												
2			17	17	22	22	25										
3			13	13	18	18	25										
4			9	9	//	//	18	18	25								
5																	
6			9	9	12	12	18	18	25								
7					6	6	12	12	23	23	25						
8	/5	15	20	20	25												
9			15	15	20	20	25										
10			12	12	15	15	22	22	25								
//			9	9	12	12	18	18	<i>2</i> 5								
12							12	12	22	22	25						
/3			12	12	14	14	22	22	25								
14					12	12	20	20	25							_	
/5			//	//	13	13	20	20	25								
16			9	9	12	12	18	18	25								
17					9	9	13	13	25	_							
18							12	12	23	23	25						
19							9	9	18	18	23	23	25				
20																	
21			8	8	//		17	17	25								
22					11		15	15	25								
23			7	7	//	//	16	16	25							_	
24					10	10	14	14	25								
25				//	13	13	20	20	25								
26			10	10	12	12	20	20	25								
27			9	9	12	12	18	18	25								
28			9	9	12	12	18	18	25								
29			9	9	12	12	18	18	25								
30			8	8	12	12	16	16	25								
3/			6	6	10	10	14	14	25								
32				_		8	12	12	25								
33			7	7	//	//	16	16	25								
34			6	6	10	10	14	14	25	_						_	
35					10 9	10 9	14	14	25 25	_						_	
36					9	9	12	12	25	21	25					_	
37							"	"	20	20	25 25	25	25				
38 39						_	9	9	18	18	23	23	25			_	
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41 42									12	12	16	16	20	20	25		
43									٠	, " <u> </u>	ن,	ļ					
43	16	16	22	22	25	<u> </u>				<u> </u>		<u> </u>				 	
45	,,,		16	16	21	21	25										
46			16	16	21	21	25										
47			16	16	21	21	25 25										
48			16	16	21	21	25										
48			14	14	18	18	25										
50 50			13	13	18	18	25										
			ر،	٠, ا	18	18	25										
5/			17	12		├─											
52			13	13	17	17	<i>2</i> 5	l		I				l		l	

COL. SIZE		2½ x ⅓														
FOUNDATION	0 x 4-0	0 x 4-3	0 x	4-9	0.	x 5-3	2-0	x 3-9	2-0	x 4-0	2-0	x 4-3	2-0	x 4-9	2-0 x	4
Sign				H	<i>EIGH</i>	T(F	eet)									
Identification Number	(+) to	(+) to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	-
53		/3	13	16	16	24	24	25								
54		13	13	16	16	24	24	25								
55		12	12	15	15	23	23	25								
56		//	//	13	13	21	21	25								
57		//	//	13	13	21	21	25								
58		//	//	/3	13	20	20	25								
59		//	//	13	13	20	20	25								
60		10	10	12	12	20	20	25								
6/		10	10	13	13	19	19	25								_
62		9	9	12	12	17	17	25								
63				12	12	17	17	25								_
64		8	8	12	12	17	17	25								
65				//	//	16	16	25								_
66				//	//	15	15	25								
67		7	7	//	//	15	15	25								
68				10	10	14	14	25								
69				10	10	14	14	25								_
70		6	6	10	10	14	14	25								_
7/																
72				9	9	14	14	25								_
73				9	9	13	13	25								
74																
75				7	7	12	12	24	24	25						_
76				7	7	12	12	23	23	25						_
77						//	//	23	23	25						_
78				7	7	12	12	24	24	25						
79																
80						10	10	19	19	23	23	25				
81						9	9	18	18	23	23	25				
82																
83																
84											ļ					
85															ļ	
86								15	15	19	19	23	23	25		_
87								/3	/3	17	17	21	21	25		_
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89							ļ				ļ		ļ		ļ	_
90					ļ		ļ				ļ		ļ		ļ	-
91																_

The Column Size is O.D. x Wall Thickness in inches.

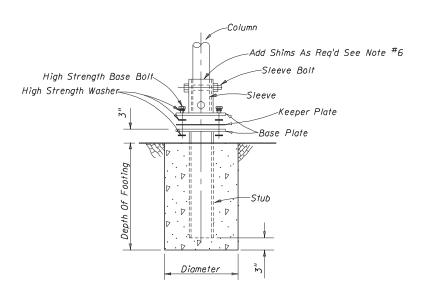
The Foundation Size is O.D. x Depth in feet & inches. A zero O.D. means that a concrete foundation is not necessary.

COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SINGLE COLUMN GROUND SIGNS

ı						
		Names	Dates	Approve	d By	12/1
	Designed By	DER	10/34	Stat	te Structures De	sign Engineer
ı	Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.
	Checked By	RES	11/94	00	2 of 2	11861



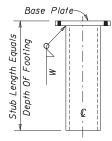
Siots - Left Shoulder

Siots - Left Shoulder

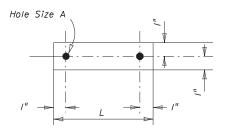
Right Shoulder

BASE DETAIL

SLEEVE & BASE PLATE DETAILS



Stub Size Equals Min. Sleeve Size Or Longer
STUB DETAIL



0.040" Thick Alum. Strip-2 Req'd Per Base
BOLT KEEPER DETAIL

SLIP BASE DETAILS

Note: Unless noted otherwise, all dimensions are in inches

· Column	S/eeve	Sleeve	Weld	Base	Plate	Radius		Bolt		It Torque	Hole
Size	I.D. (Max)	Height H	W	L	T	R	Size	Length	Ft-lbs	In-Ibs	Size A
4 x ½	4 <u>/</u> 6	6	5 8	8	<u>3</u>	<u> </u> 32	<u>5</u> 8	3	29	355	<u>//</u> 6
$4\frac{1}{2} \times \frac{1}{4}$	4 <u>9</u>	6	<u>5</u> 8	8	<u>7</u>	<u>11</u> 32	<u>5</u>	3 <u>/</u>	29	355	<u> 11</u>
5 x ½	5 <u>/</u> 6	7	<u>5</u> 8	8	<u>7</u>	<u> </u> 32	<u>5</u>	3 <u>/</u>	29	<i>3</i> 55	<u>//</u>
6 x ½	6 <u>/</u> 6	8	<u> </u> 6	9	1	<u>7</u> 16	<u>3</u>	3 <u>/</u>	48	580	<u>/3</u> /6
8 x <u>5</u>	8 <u>/</u>	10	<u>3</u>	11	1	<u> </u>	<u>7</u>	3 3	53	640	<u>15</u> 16

NOTES

- I. Work this Standard with Standard Index Numbers 11860 and 11865.
- 2. To determine column (post) size and footing requirements use the required Sign Identification Number and Sign Height (H), Designs for Heights (H) lower than those listed in the Table are included in Standard Index Number II865.
- 3. Single Column installations are not allowed for heights (H) exceeding the maximum height shown in the Table, and for sign profiles (Sign Identification Numbers) without any design tablulated. In this event, the sign(s) will have to be supported by multiple columns (posts) featuring breakaway devices. See Standard Index Number 9535.
- 4. The Column (Post) material shall be aluminum. The size is given as outside diameter and wall thickness. Columns (posts) larger than 3½"x½" are nonfragible and shall be installed with breakaway supports and will have concrete footings and slip bases.
- 5. The foundation size is given as outside diameter and depth.
 - a) Frangible Supports: Foundations for Frangible Supports do not require concrete.

 The column (post) shall be driven into the ground to the depth indicated.
 - b) Breakaway Supports: Foundations for Breakaway Supports require concrete. The column support shall be set in a concrete foundation, sized as shown in the table. The first dimension indicates the diameter and the second dimension the depth into the ground. In all cases the ground is to be considered as undisturbed earth, road material, or properly compacted fill.
- 6. SLIP BASE NOTES :
 - a) The Inside Diameter (I.D.) of the sleeve shall be no more than $\frac{L}{16}$ " larger than the Outside Diameter (O.D.) of the Column.
 - b) The sleeve bolts shall be $\frac{1}{2}$ " Ø with locknuts. The bolts shall be galvanized steel (ASTM A-307) or Aluminum Association Alloy 2024-T4 or 6061-T6 (ASTM B-211).
 - c) The base bolts, nuts and washers shall be high strength ASTM A-325 and shall have an electroplated zinc coating SC3, Type II applied in accordance with ASTM B633.
 - d) An alternate cast base of aluminum alloy 356 and T6 temper in lieu of the fabricated base may be submitted for approval by the Engineer. If a cast base is used the stub will be the same as the column and will be bolted to the casting.
 - e) Assemble the slip base connection in the following manner:
 Connect column to sleeve using two (2) ½"Ø machine bolts.
 Assemble top base plate to stub base plate using high strength bolts with three (3) hardened washers per bolt. One (1) washer per bolt and two (2) bolt keeper plates go between the base plates.
 Use shim stock as required to plumb the column.
 Tighten all bolts the maximum possible with a 12" to 15" wrench to bed the washers and shims and to clear the bolt threads. Loosen each bolt one (1) turn and retighten to the prescribed torque (see table). Bolts shall be tightened with properly calibrated wrenches under the supervision of the project engineer.
 - Burr threads at junction with nut using a center punch to prevent nut loosening.

 f) Use galvanized steel shims to obtain a tight fit between the column face and the sleeve. Place shims in all quadrants between the ½" Ø sleeve bolts.

 The shim length shall be !" shorter than the height of the sleeve.

COLUMN SIZE, COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SINGLE COLUMN GROUND SIGNS

1 of 2

COL. SIZE	2 x ½	2½ x ½	3 x ⅓	3 ½	x <u>3</u>	4 /	x <u>3</u>	4 <u>/</u> 2 .	χ <u>/</u>	5 >	1 <u>1</u>	6 x	4	8.	x <u>16</u>
FOUNDATION	0 x 4-3	0 x 4-3	0 x 4-9	0 x	6-0	2-0	x 4-0	2-0 x	4-0	2-0	x 4-3	2-0 x	5-0	2-0 x	: 5-0
Sign Identification					HEIGH	IT (I	Feet)								
Number		(+) to	(+) to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to
/		16	16 20	20	25										
2			17	17	24	24	25								
3			13	13	20	20	25								
4		6	6 9	9	13	/3	25								
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6			10	10	13	13	25								
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8		16	16 20		25										
9			/5	15	22		25								
10			12	12	17	17	25								
//			9	9	13		25								
12					8	8	16	16	21	21	25				
/3			12	12	17		25								
14			//	//	15	15	25								
/5			12	12	15		25								
16			10	10	13	13	25								
17					//	//	20	20	25						
18					10	10	18		22		25				
19							14	14	17	17	21	21	25		
20															
21		6	6 9	9	13	13	25								
22			7	7	//	//	23	23	25						
23			8	8	12	12	23		25						
24					//	//	21	21	25						
25			12	12	16	16	25								
26			//	//	15	15	25								
27			10	10	14	14	25								
28			10	10	14	14	25								
29			10	10	13	13	25								
30			8	8	12	12	24		25						
3/			7	7	12	12	21	21	25	04	05				
32			6	6	//	//	20	20	24	24	25				
33			7	8 7	12	12	23	23	25						
34			7	7	"	"	21	21 22	25 25						
35 36			6	6	"	// //	22 20	20	23	24	25				
37				<u> </u>	8	8	16	16	20	20	23	24	25		
38					7	7	14	14	19	19	23	23	25		
39					6	6	/3	/3	17	17	21	21	25		
40							, ,								
41							//		14	14	17	17	25		
42							10	10	12	12	15	15	22	22	25
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44		17	17 21	21	25										
45		· · · · ·	16	16	23	23	25								
46			16	16	23	23	25								
47			16	16	23	23	25								
48			16	16	23	23	25								
49			14	14	21	21	25								
50			14	14	20	20	25								
51			14	14	20	20	25								
52			/3	13	20	20	25								
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COL. SIZE	2 x ½	2½ x ⅓	3 x ½	3½	x 3/6	4	х <u>з</u>	4 <u>1</u>	x ¼	5)	(<u> </u>	6	х <u>I</u>	8 x	<u>5</u> /6
FOUNDATION	0 x 4-3	0 x 4-3	0 x 4-9	0 x	6-0	2-0	x 4-0	2-0	x 4-0	2-0	x 4-3	2-0	x 5-0	2-0)	x 5-0
Sign			H	IEIGH :	T (F	eet)									
Identification Number	(+) to	(+) to	(+) to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to	(+)	to
53			13	13	19	19	25								
5 <i>4</i>			12	12	18	18	25								
55			12	12	18	18	25								
56		8	8 12	12	17	17	25								
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58			12	12	16	16	25								
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60			//	//	19	19	25								
6/			//	//	15	<i>1</i> 5	25								
62			9	9	13	13	25								
63			9	9	13	13	25								
64			9	9	12	12	24	24	<i>2</i> 5						
65			8	8	12	12	23	23	25						
66					12	12	23	23	25						
67			7	7	12	12	22	22	25						
68			7	7	12	12	22	22	<i>2</i> 5						
69			7	7	12	12	22	22	25						
70			6	6	//	//	21	21	<i>2</i> 5						
71															
72					12	12	21	21	<i>2</i> 5						
73			6	6	//	//	20	20	24	24	25				
74															
75					10	10	18	18	22	22	<i>2</i> 5				
76					10	10	18	18	22	22	<i>2</i> 5				
77					9	9	17	17	22	22	<i>2</i> 5				
78					10	10	18	18	22	22	<i>2</i> 5				
79															
80							14	14	18	18	22	22	<i>2</i> 5		
81							13	/3	17	17	21	21	25		
82															
83															
84															
85															
86							//	//	14	14	17	17	<i>2</i> 5		
87							//	//	12	12	16	16	23	23	2
88															
89															
90				ļ											
91															

The Column Size is O.D. x Wall Thickness in inches.

The Foundation Size is 0.D. x Depth in feet & inches. A zero 0.D. means that a concrete foundation is not necessary.

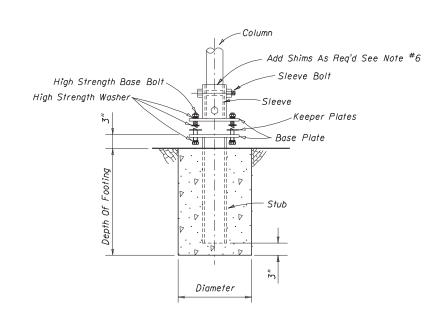
COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SINGLE COLUMN GROUND SIGNS

70 WIND LOADING

ı						
		Names	Dates	Approve	d By	12/1
	Designed By	DER	10/34	Stat	te Structures De	sign Engineer
ı	Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.
	Checked By	RES	11/94	00	2 of 2	11862



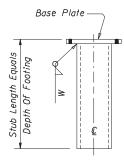
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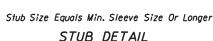
Slots - Left Shoulder

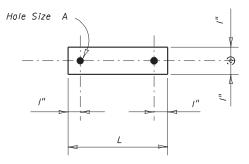
Slots - Right Shoulder

SLIP BASE AND FOOTING DETAIL

SLEEVE & BASE PLATE DETAILS







0.04" Thick Alum. Strip-2 Req'd Per Base
BOLT KEEPER DETAIL

SLIP BASE DETAILS

Column	Sleeve	S/eeve	Weld	Base I	Plate	Radius	Base	Bolt	Base Bo	It Torque	Hole
Size	I.D. (Max)	Height H	W	L	T	R	Size	Length	Ft-lbs	In - Ibs	Size A
4 x 1/4	4 <u>/</u> 6	6	<u>5</u>	8	<u>3</u>	<u>11</u> 32	5/8	3	29	355	<u>//</u>
4½ x ¼	4 <u>9</u>	6	<u>5</u>	8	7/8	<u>//</u> 32	5 8	3 ½	29	355	<u>//</u>
5 x ½	5 <u>/</u> 6	7	<u>5</u> 8	8	<u>7</u>	<u>//</u> 32	<u>5</u> 8	3 <u>/</u>	29	355	<u>//</u>
6 x ½	6 <u>/</u>	8	<u> </u>	9	/	<u>7</u> 16	34	3 <u>/</u>	48	580	<u>13</u> 16
8 x 5	8 <u>/</u> 6	10	<u>3</u>	//	1	<u> </u>	<u>7</u>	3 3	53	640	<u>15</u> 16

Note: Unless noted otherwise, all dimensions are in inches.

NOTES

- I. Work this Standard with Standard Index Numbers 11860 and 11865.
- 2. To determine column (post) size and footing requirements use the required Sign Identification Number and Sign Height (H), Designs for Heights (H) lower than those listed in the Table are included in Standard Index Number II865.
- 3. Single Column installations are not allowed for heights (H) exceeding the maximum height shown in the Table, and for sign profiles (Sign Identification Numbers) without any design tablulated. In this event, the sign(s) will have to be supported by multiple columns (posts) featuring breakaway devices. See Standard Index Number 9535.
- 4. The Column (Post) material shall be aluminum. The size is given as outside diameter and wall thickness. Columns (posts) larger than 3"x $\frac{1}{8}"$ are non-fragible and shall be installed with breakaway supports and will have concrete footings and slip bases.
- 5. The foundation size is given as outside diameter and depth.
 - a) Frangible Supports: Foundations for Frangible Supports do not require concrete.

 The column (post) shall be driven into the ground to the depth indicated.
 - b) Breakaway Supports: Foundations for Breakaway Supports require concrete. The column support shall be set in a concrete foundation, sized as shown in the table. The first dimension indicates the diameter and the second dimension the depth into the ground. In all cases the ground is to be considered as undisturbed earth, road material, or properly compacted fill.

6. SLIP BASE NOTES :

- a) The Inside Diameter (I.D.) of the sleeve shall be no more than $\frac{I}{I6}$ " larger than the Outside Diameter (O.D.) of the Column.
- b) The sleeve bolts shall be 12 Ø with locknuts. The bolts shall be galvanized steel (ASTM A-307) or Aluminum Association Alloy 2024-T4 or 6061-T6 (ASTM B-211).
- c) The base bolts, nuts and washers shall be high strength ASTM A-325 and shall have an electroplated zinc coating SC3, Type II applied in accordance with ASTM B633.
- d) An alternate cast base of aluminum alloy 356 and T6 temper in lieu of the fabricated base may be submitted for approval by the Engineer. If a cast base is used the stub will be the same as the column and will be bolted to the casting.
- e) Assemble the slip base connection in the following manner:

Connect column to sleeve using two (2) $\frac{1}{2}$ " Ø machine bolts. Assemble top base plate to stub base plate using high strength bolts with three (3) hardened washers per bolt. One (1) washer per bolt and two (2) bolt keeper plates go between the base plates.

Use shim stock as required to plumb the column.

Tighten all bolts the maximum possible with a 12" to 15" wrench to bed the washers and shims and to clear the bolt threads. Loosen each bolt one (I) turn and retighten to the prescribed torque (see table). Bolts shall be tightened with properly calibrated wrenches under the supervision of the project engineer.

Burr threads at junction with nut using a center punch to prevent nut loosening. f) Use galvanized steel shims to obtain a tight fit between the column face and the sleeve. Place shims in all quadrants between the $\frac{1}{2}$ " Ø sleeve bolts. The shim length shall be l" shorter than the height of the sleeve.

COLUMN SIZE, COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SINGLE COLUMN GROUND SIGNS

80 WIND LOADING

	Names	Dates	Approve	d By	12/1
signed By	DER	10/34	Stat	te Structures De	sign Engineer
awn By	DDDS	10/94	Revision	Sheet No.	Index No.
ecked By	RES	11/94	00	1 of 2	11863

				_								_			
COL. SIZE		2½ x ½				1									
FOUNDATION	0 x 4-6	0 x 4-9	0 x 4-9	0)				2-0	x 4-0	2-0	x 4-3	2-0	x 5-0	2-0 x	5-6
Sign Identification					HEIGI	т —									
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2			14	14	20	20	25								
3			_	+-		17	25								
4			7	7	//	//	21	21	25			_		_	
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14				+	12	12	23	23	25						
/5				+	12	12	24	24	25						
16				+	12	12	21	21	25						
17				+	8	8	16	16	20	20	24	24	25		
18							14	14	18	18	22	22	25		
19							//	//	14	14	17	17	25		
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21			7	7	//	//	21	21	25						
22				1	10	10	18	18	23	23	25				
23					//	//	19	19	23	23	25				
24					9	9	17	17	21	21	25				
25					12	12	24	24	25						
26					12	12	23	23	25						
27					12	12	21	21	<i>2</i> 5						
28					12	12	22	22	<i>2</i> 5						
29					12	12	21	21	25						
30					//	//	20	20	24	24	25				
3/					9	9	17	17	21	21	25				
32					8	8	16	16	20	20	24	24	<i>2</i> 5		
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34				1	9	9	17	17	21	21	25				
3 5				1	10	10	18	18	22	22	25				
36					8	8	16	16	20	20	24	24	<i>2</i> 5		
37				1			12	12	16	16	20	20	25		
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55				14	14	25		\dashv						
56			9	9 /3	13	25		\dashv						
57			,	/3	13	24	24 2	25						
58 58				12	12	24		25						
59				12	12	23		25						
60				12	12	23		25						
6/			8	8 13	13	22		25						
62				12	12	21		25						
63				12	12	21		25						
64			6	6 12	12	20		24	24	25				
65				//	//	19		24	24	25				
66				"	//	19		23	23	25				
67				10	10	18		22	22	25				
68				10	10	18		22	22	25				
69				10	10	18		22	22	25				
70				9	9	17		2/	21	25				
71														
72				9	9	17	17 2	2/	21	25				
73				8	8	16	16 2	20	20	24	24	25		
74														
75				7	7	14	14 1	8	18	22	22	25		
76				7	7	14		8	18	22	22	25		
77						/3	13 1	17	17	21	21	25		
78				7	7	14	14 1	8	18	22	22	25		
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The Column Size is O.D. x Wall Thickness in Inches

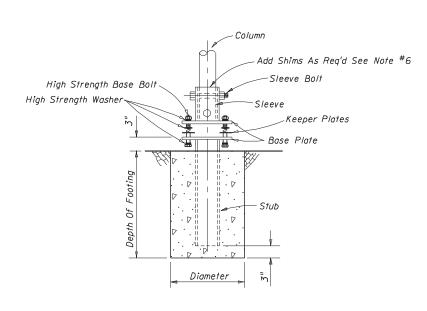
The Foundation Size is O.D. x Depth in feet & Inches. A zero O.D.means that a concrete foundation is not necessary.

COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SINGLE COLUMN GROUND SIGNS

	Names	Dates	Approve	d By	12/				
Designed By	DER	10/94	State Structures Design Engineer						
Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.				
Checked By	RES	11/94	00	2 of 2	11863				



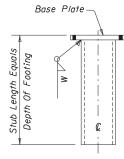
Slots - Left Shoulder

Slots - Right Shoulder

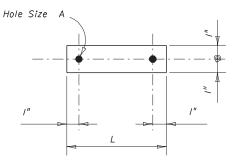
Slots - Right Shoulder

SLIP BASE AND FOOTING DETAIL

SLEEVE & BASE PLATE DETAILS







0.04" Thick Alum. Strip-2 Req'd Per Base BOLT KEEPER DETAIL

SLIP BASE DETAILS

Column	S/eeve	Sleeve	Weld	Base	Plate	Radius	Base	Bolt	Base Bo	It Torque	Hole
Size	I.D. (Max)	Height H	W	L	T	R	Size	Length	Ft-lbs	In - Ibs	Size A
4 x ½	4 <u>/</u>	6	<u>5</u> 8	8	<u>3</u>	<u>11</u> 32	<u>5</u> 8	3	29	3 55	<u>//</u>
$4\frac{1}{2} \times \frac{1}{4}$	4 <u>9</u>	6	<u>5</u>	8	<u>7</u>	<u> </u> 32	<u>5</u>	3 <u>/</u>	29	355	<u>//</u> 6
5 x ½	5 <u>/</u>	7	<u>5</u> 8	8	78	<u>//</u> 32	<u>5</u>	34	29	<i>3</i> 55	<u>//</u> 6
6 x ½	6 <u>/</u>	8	<u>//</u> 6	9	/	<u>7</u> 16	<u>3</u>	3 <u>/</u>	48	580	<u>/3</u> /6
8 x <u>5</u>	8 <u>/</u> 6	10	<u>3</u>	//	1	<u>/</u> 2	<u>7</u>	3 <u>3</u>	53	640	<u>/5</u> /6

Note: Unless noted otherwise, all dimensions are in inches.

NOTES

- I. Work this Standard with Standard Index Numbers 11860 and 11865.
- 2. To determine column (post) size and footing requirements use the required Sign Identification Number and Sign Height (H), Designs for Heights (H) lower than those listed in the Table are included in Standard Index Number II865.
- 3. Single Column installations are not allowed for heights (H) exceeding the maximum height shown in the Table, and for sign profiles (Sign Identification Numbers) without any design tablulated. In this event, the sign(s) will have to be supported by multiple columns (posts) featuring breakaway devices. See Standard Index Number 9535.
- 4. The Column (Post) material shall be aluminum. The size is given as outside diameter and wall thickness. Columns (posts) larger than $3\frac{1}{2}$ " x $\frac{1}{8}$ " are non-fragible and shall be installed with breakaway supports and will have concrete footings and slip bases.
- 5. The foundation size is given as outside diameter and depth.

of the project engineer.

- a) Frangible Supports: Foundations for Frangible Supports do not require concrete.

 The column (post) shall be driven into the ground to the depth indicated.
- b) Breakaway Supports: Foundations for Breakaway Supports require concrete. The column support shall be set in a concrete foundation, sized as shown in the table. The first dimension indicates the diameter and the second dimension the depth into the ground. In all cases the ground is to be considered as undisturbed earth, road material, or properly compacted fill.

6. SLIP BASE NOTES :

- a) The Inside Diameter (I.D.) of the sleeve shall be no more than $\frac{L}{16}$ larger than the Outside Diameter (O.D.) of the Column.
- b) The sleeve bolts shall be $\frac{1}{2}$ " Ø with locknuts. The bolts shall be galvanized steel (ASTM A-307) or Aluminum Association Alloy 2024-T4 or 6061-T6 (ASTM B-211).
- c) The base bolts, nuts and washers shall be high strength ASTM A-325 and shall have an electroplated zinc coating SC3, Type II applied in accordance with ASTM B633.
- d) An alternate cast base of aluminum alloy 356 and T6 temper in lieu of the fabricated base may be submitted for approval by the Engineer. If a cast base is used the stub will be the same as the column and will be bolted to the casting.
- e) Assemble the slip base connection in the following manner:
 Connect column to sleeve using two (2) ½" Ø machine bolts.
 Assemble top base plate to stub base plate using high strength bolts with three (3) hardened washers per bolt. One (1) washer per bolt and two (2) bolt keeper plates go between the base plates.
 Use shim stock as required to plumb the column.
 Tighten all bolts the maximum possible with a 12" to 15" wrench to bed the washers and shims and to clear the bolt threads. Loosen each bolt one (1) turn and retighten to the prescribed torque (see table). Bolts shall be tightened with properly calibrated wrenches under the supervision
- Burr threads at junction with nut using a center punch to prevent nut loosening.

 f) Use galvanized steel shims to obtain a tight fit between the column face and
 the sleeve Place shims in all guarants between the $\frac{1}{2}$ " @ sleeve bolts.

the sleeve. Place shims in all quadrants between the $\frac{1}{2}$ " Ø sleeve bolts. The shim length shall be 1" shorter than the height of the sleeve.

COLUMN SIZE, COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SINGLE COLUMN GROUND SIGNS

	Names	Dates	Approve	d By/)	12/1				
gned By	DER	10/34	State Structures Design Engineer						
vn By	DDDS	10/94	Revision	Sheet No.	Index No.				
cked By	RES	11/94	00	1 of 2	11864				

	0 /		- /	- 1	7		,	. 1	,		,	_		_	5 1
COL. SIZE	2 x ½	2½ x ½					x ½				x 1/4			8 x	
	0 x 4-6	0 x 4-9	0 x 4-9						X 4-0	2-0	X 4-3	2-0	x 5-0	2-0 2	r 6-0
Sign Identification						_	Feet)	_		_					
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7							12	12	14	14	18	18	<i>2</i> 5		
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14					//	//	19	19	23	23	25				
/5					12	12	20	20	24	24	25				
16					10	10	18	18	22	22	25				
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25					12	12	20	20	24	24	25				
26					11	11	19	19	23	23	25				
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27					10		18	18	22						
28					10	10	18	18		22	25				
29					10	10	17	17	21	21	25				
30					9	9	16	16	20	20	24		25		
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32					6	6	/3	/3	16	16	20		25		
33					8	8	16	16		19		24	25		
34					7	7	14	14	18	18		22	25		
35					7	7	14	14	18	18	22	22	25		
36					6	6	/3	13	16	_	20	20	25		
37							//	//	13	13		16		24	25
38							//	//	12	12	/5	15		22	25
39							10	10	//	//	13	13	20	20	25
40															
41									10	10	//	//	16	16	25
42											10	10	14	14	<i>2</i> 5
43															
44			16	16	21	21	<i>2</i> 5								
45					17	17	<i>2</i> 5								
46					17	17	<i>2</i> 5								
47					16	16	<i>2</i> 5								
48					16	16	25								
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52		1			14	14		24	25						
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COL. SIZE	2 x 1	2½ x ½	3 x ½	3/2 x 3	4 y .	<u> </u>	4 / .	x 1/4	5	<u>, </u>	6	x 1/4	8 x	<u> 5</u>
		0 x 4-9												
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58				12	12	20	20	24	24	25				
59				12	12	20	20	24	24	25				
60				//	//	19	19	23	23	25				
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62				10	10	17	17	21	21	25				
63				10	10	17	17	21	21	25				
64				9	9	17	17	21	21	25				
65				8	8	16	16	20	20	24	24	25		
66						15	15	19	19	23	23	25		
67				8	8	15	15	19	19	23	23	25		
68				7	7	14	14	18	18	22	22	25		
69				8	8	14	14	18	18	22	22	25		
70				7	7	14	14	18	18	22	22	25		
71														
72						14	14	17	17	21	21	25		
73				6	6	13	13	16	16	20	20	25		
74														
75						12	12	15	15	19	19	25		
76						12	12	15	15	18	18	25		
77						//	//	14	14	17	17	25		
78						12	12	/5	/5	19	19	25		
79							ļ							
80						10	10	12	12	14	14	21	21	25
81						9	9	//	//	13	13	20	20	25
82														
83														
84														
85														
86								10	10	//	//	17	17	25
87										//	//	/5	/5	25
88														
89														
90							ļ							
91					l .		l .							

The Column Size is O.D. x Wall Thickness in inches.

The Foundation Size is O.D. x Depth in feet & inches. A zero O.D.means that a concrete foundation is not necessary.

COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

SINGLE COLUMN GROUND SIGNS

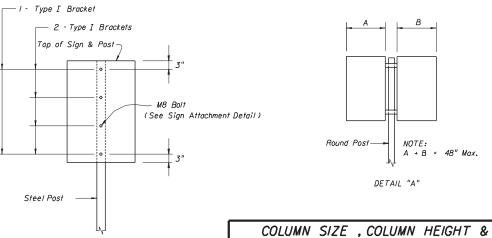
	Names	Dates	Approve	Approved By									
Designed By	DER	10/34	State Structures Design Engineer										
Drawn By	DDDS	10/94	Revision	Sheet No.	Index No.								
Checked By	RES	11/94	nn	2 of 2	<i> 1864</i>								

	,	,	,		1 1	, ,	,	,	,		П	,	,	,	, ,	, ,	, ,	,	,	,		
COL. SIZE	2x ½	2x 1/8	2x ½	$2\frac{1}{2}x\frac{1}{8}$	2½x½	2½x½	3x 8	3x ½	3x / 8	*	COL. SIZE	2x 1/8	2x ½	2x 1/8	2½x½	2½x½	2½x½	3x ½	3x ¹ / ₈	3x 1/8	*	ALUMINUM
FOUNDATION	0x2-0	0x2-0	0x2-0	0x2-3	0x2-3	0x2-3	0x2-6	0x2-6	0x2-6	*	FOUNDATION		0x2-0					0x2-6	0x2-6	0x2-6	*	ROUND POST
COL. SIZE	2.5 #/FT	2.5 #/FT	3.0 #/FT	4.0 #/FT	4.0 #/FT	N/A	N/A	N/A	N/A	*	COL. SIZE	2.5 #/FT	2.5 #/FT	3.0 #/FT	4.0 #/FT	4.0 #/FT	N/A	N/A	N/A	N/A	*	STEEL FLANGED
FOUNDATION	0x3-0	0x3-0	0x3-0	0x3-0	0x3-0	N/A	N/A	N/A	N/A	*	FOUNDATION	0x3-0	0x3-0	0x3-0	0x3-0	0x3-0	N/A	N/A	N/A	N/A	*	CHANNEL POST
COL. SIZE	W = /1/2	$W = /\frac{3}{4}$	W = /3/4	W = 2	W = 21/4	W = 21/4	W = 21/4	W = 2½	N/A	*	COL. SIZE	W = /1/2	$W = /\frac{3}{4}$	$W = /\frac{3}{4}$	W = 2	W = 21/4	W = 21/4	W = 21/4	W = 2½	N/A	*	STEEL SQUARE
FOUNDATION		0x3-0	0x3-0		<u> </u>		0x3-0	0x3-0		*	FOUNDATION		0x3-0	0x3-0			0x3-0	0x3-0	0x3-0	N/A	*	TUBE POST
Sign											Sign											
Identification					HEIGHT						Identification					HEIGHT						
	(+) to	(+) to	(+) to	(+) to	(+) to	(+) to	(+) to	(+) 10	(+) 10	(+) to	Number					(+) to			(+) to	(+) to	(+) to	1
1	To 8		8 - 10	10 - 13			13 - 14				47	To 6	6 - 7	7 - 9	9 - 11	11 - 12	12 - 13	13 - 14				/.
2	To 6	6 - 7	7 - 8	8 - 12		12 - 13	13 - 14				48		To 7	7 - 8	8 - 11	11 - 12		12 - 13	13 - 14			ĺ
3		To 6	6 - 7	7 - 9	9 - 11		11 - 12	12 - 13			49		To 6	6 - 7	7 - 10	10 - 11		11 - 12	12 - 14			1
4											50											1
5											5/		To 6	6 - 7	7 - 9	9 - 11		11 - 12	12 - 14			2.
6					To 6		6 - 8	8 - 9			52			To 6	6 - 9	9 - 10		10 - 12	12 - 13			
7											53			To 6	6 - 8	8 - 10		10 - 11	11 - 13			. 3.
8	To 8	8 - 9	9 - 10	10 - 13		13 - 14					54				To 8	8 - 9		9 - 11	11 - 12			4.
9		To 7	7 - 8	8 - 11	11 - 12	12 - 13		13 - 14			55				To 8	8 - 9		9 - 10	10 - 12			
		70 7	7 - 0								1				70 0			3 ,0				5
10				To 8	8 - 9 To 6		9 - 10	10 - 12			56				T	<u> </u>		7 - 9	9 - 10			6.
//					1,00		6 - 7	7 - 9			57				To 7							7.
12											58				To 6	6 - 7		7 - 9	9 - 11			8.
13					To 8		8 - 9	9 - 11	11 - 12		59				To 6	6 - 7		7 - 9	9 - 10			
14				To 6	6 - 7		7 - 8	8 - 10	10 - 11		60				To 6	6 - 7		7 - 8	8 - 10			1
15				To 6	6 - 7		7 - 8	8 - 10			61				To 6			6 - 8				9.
16					To 6		6 - 7	7 - 9			62					To 6		6 - 7	7 - 8			j.
17											63					To 6		6 - 7	7 - 9			
18											64											IC.
19											65							To 6	6 - 8			//-
20											66											12
21							To 6				67							To 6	6 - 7			/3
22											68									To 7		14
23								To 6			69								To 7			1
24											70								To 6			
25				To 6	6 - 7		7 - 9	9 - 11			71											ĺ
26				To 6			6 - 8	8 - 9			72											1
27					To 6		6 - 7	7 - 9			73								To 6			
28					To 6		6 - 7	7 - 9			74											<u>.</u>
29							To 7	7 - 8			75											1
30							To 6	6 - 8			76											1
31								To 6			77											
32								To 6			78											
33							To 6	6 - 7			79											
34								To 6			80											1
35								To 7			81											
36								To 6			82											
37											83											1
38											84											ĺ
39											85											
40											86											SIGI
40											87											3101
											1											
42											88											
43						17					89											
44	To 9			10 - 13		13 - 14	13 14				90											
45 46		6 - 7			11 - 12 11 - 12						91											
70		U - 1	1, , ,	3 - 11	11 - 12	ال ت ال	1.5 17			<u> </u>	JL						1	l	L	1		ı
I																						

- * Aluminum Round Post dimensions are given in inches. The size is shown as outside diameter times wall thickness.
- Steel Flanged Channel Post sizes are given in lb/ft. Section definitions and properties are shown on Sheet 2 of 2, (See OPL for approved posts).
- Steel Square Tube Post dimensions for "W" are given in inches. The "W" dimension is defined on Section F-F, (See QPL for approved posts).
- Foundation dimensions shown are given in feet & inches. The dimension shown is the minimum embedment of the driven post.

NOTES

- I. This Standard Index II865 provides designs for driven single post sign installations for implementation a<u>t all locations with</u>in the State of Florida. The designs adhere to the following criteria:
 - a. Mounting Height = 14' Maximum b. Sign(s) Area = 25 sq. ft. Maximum
 - c. Sign(s) Width: Single = 36" Maximum
 - Dual = 48" Maximum (See Detail "A")
 - d. Driven Post only
- 2. Designs exceeding above criteria or requiring concrete footings are included on Index II864.
- 3. Specifications for Aluminum materials, Sign Panel Details, etc. are shown on standard Index II860. Additional information and details are shown on Index II861 thru II864. Therefore, work this Standard Index II865 with Standard Indices II860 to II864.
- 4. Sign Bracket requirements are shown on Index II860 (80 mph WIND ZONE). If Flanged Channels or Square Tubes are used, substitute two 5% bolts for each Type I Bracket. See Detail B.
- 5. All posts shall be installed Plumb.
- 6. Steel for Flanged Channel Posts shall conform with ASTM A499 Grade 60. or ASTM A576 Grade 1080.
- 7. Steel for fabrication of square Tubes shall conform with ASTM A653 or ASTM A570. HOWEVER, <u>STEEL FROM</u> THE FABRICATED SQUARE TUBES MUST MEET A CERTIFIED MINIMUM YIELD STRENGTH OF 55 ksi.
- 8. Steel Flanged Channel Posts with a 4 lb/ft are non-frangible and shall be installed with approved breakaway (frangible) bases. See Detail "C". The base and the sign posts shall be same size and type and the splice shall be 6" long and fastened with two bolts, 4" apart. The bolts shall be wrench-tightened sufficiently to clamp splice assembly tightly tagether. Bolts shall conform with ASTM A 354 Grade DH or SAE J995 Grade 8. Washers and spacers shall conform with ASTM A307 or A36.
- Steel Flanged Channel Posts with masses of 2.5 lb/ft and 3 lb/ft, all Aluminum Round Posts and all Steel Square Tubes
 included in this standard are frangible and do not require breakaway (frangible) bases. However, the contractor may mount
 frangible posts on approved breakaway bases.
- 10. Bolts, Nuts and washers not included in note 8 above, shall conform with ASTM A307.
- II. Steel Posts shall be selected from the Department's book of Qualified Product List (QPL).
- I2. All steel posts, and hardware shall be galvanized in accordance with ASTM Al23 or Al53, or AASHTO MI8I Grade 2.
- 13. Shop Drawings: If the contractor proposes to utilize sign panel connections and/or breakaway devices not shown in this standard or in the above referenced standards, the Contractor shall submit shop drawings for approval.
- 14. All dimensions are in inches, unless otherwise noted.



DETAIL "B" (See Note No. 4)

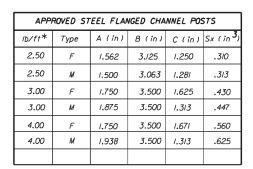
SIGN MOUNTING USING CHANNELS OR SQUARE TUBES

COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

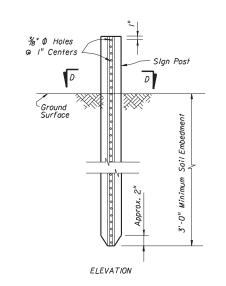
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

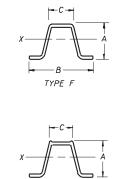
SINGLE COLUMN GROUND SIGNS

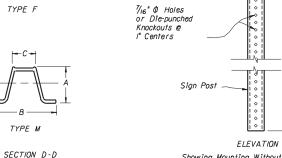
			Names	Dates	Approve	d By /)	12/
ı		Designed By	JMD	4-94	Stat	e Structures De	sign Engineer
ı	HEIGHT = 14' MAX.	Drawn By	SHM	4-94	Revision	Sheet No.	Index No.
	(ALL WIND ZONES)	Checked By	AJG	4-94	00	1 of 2	11865



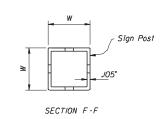


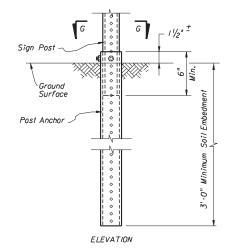




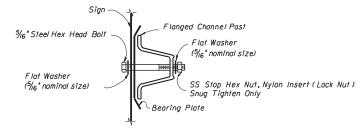


Showing Mounting Without Anchor Tube

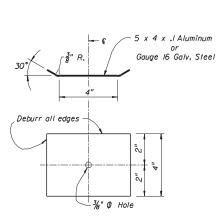




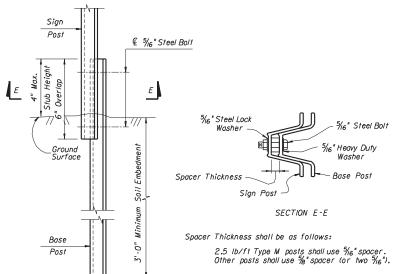
Showing Mounting Using Optional Anchor Tube



SIGN ATTACHMENT DETAIL

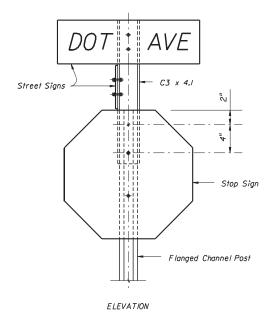


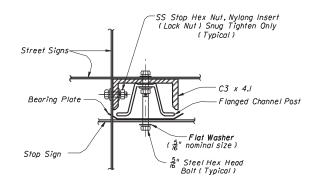
BEARING PLATE DETAIL



STEEL FLANGED CHANNEL POST DETAILS

DETAIL "C" (Approved Frangible Installation)

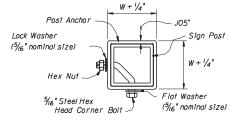




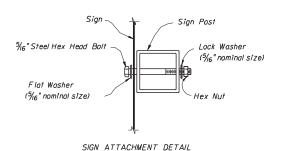
SIGN ATTACHMENT DETAIL

TOP VIEW

NOTE: All dimensions are in inches, unless otherwise noted.



SECTION G-G



STEEL SQUARE TUBE POST DETAILS

COLUMN SIZE , COLUMN HEIGHT & COLUMN FOOTINGS

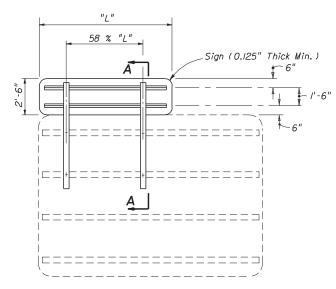
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

SINGLE COLUMN GROUND SIGNS

Names Dates Approved By / Designed By HEIGHT = 14' MAX. Checked By 00 2 of 2

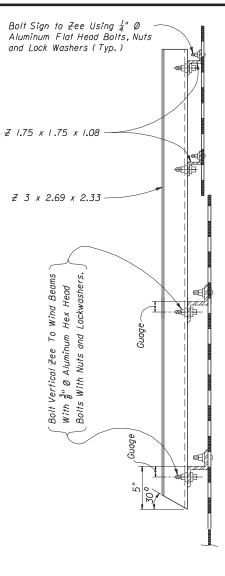
(ALL WIND ZONES)

\$\$\$\$\$\$\$\$YTIME\$\$\$\$\$\$



NOTE: Exit numbering panel shall be located to the right side for right exit and to the left for left exit.

Mounting of Exit Numbering Panels To Highway Signs **ELEVATION**



SECTION AA

GENERAL NOTES

DESIGN SPECIFICATION: Latest Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals, AASHTO 1994.

SHEETS AND PLATES: Material used shall meet the requirements of Aluminum Association Alloy 6061-T6 and ASTM B209. Sheets are to be degreased, etched, neutralized and treated with Alodine II200, Iridite I4-2 Bonderite 721, or equal. No stenciling permitted on sheets.

MATERIALS: All aluminum materials shall meet the requirements of the Aluminum Association Alloy 6061-T6 and also the following ASTM specifications for the following: Sheets and plates B209; extruded shapes B221 and standard structural shapes B308.

ALUMINUM BOLTS, NUTS & LOCK WASHERS: Aluminum bolts shall meet the requirements of the Aluminum Association Alloy 2024-T4 or 606I-T6 (ASTM B2II). The bolts shall have an anodic coating of at least .0002" thick and be chromate sealed. Lockwashers shall meet the requirement of Aluminum Association Alloy 7075-T6 (ASTM B22I). Nuts shall meet the requirement of Aluminum Association Alloy 6262-T9 or 606I-T6.

SIGN FACE: All sign face corners shall be rounded . See sign layout sheet for dimension "L" and sign face details.

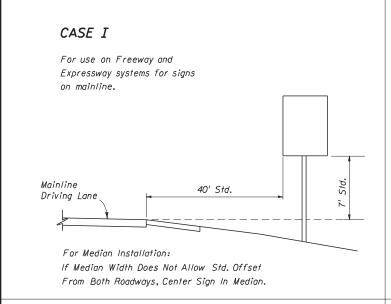
MATERIAL STRESSES: All allowable stresses are in accordance with Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. AASHTO for all materials shown in the plans.

For mounting details refer to Index No. 11037.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN

MOUNTING EXIT NUMBERING PANELS TO HIGHWAY SIGNS

	Names	Dates	Approve	d By /) 7	12/1				
Designed By	CK/CWB	7-82	State Structures Design Engineer						
Drawn By			Revision	Sheet No.	Index No.				
Checked By	CK	7-82	00	I of I	13417				



CASE II

CASE V

For Use In All Rural Roads And
On Freeway And Expressway Ramps.

12' Min.

12' Min.

Paved Shoulder

14' Horizontal Clearance Standard On All Freeway And Expressway Ramps

For Sections Without Paved Shoulder The 6' Min Does Not Apply.

For Use In Business Or Residential Areas Only.

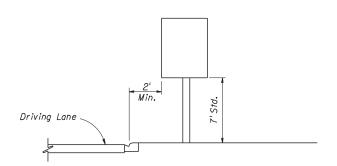
CASE III

For Use On All Roads With Signs Mounted Behind Sidewalk.

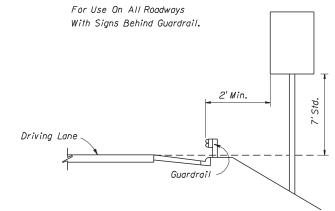
2' Min.

Curb & Gutter Sidewalk

Driving Lane



CASE VI



CASE VII (REST AREA & EXIT GORE SIGNS)

For Use On All Freeway And Expressway Systems

CASE IV (Merge Sign)

14' Min.

Mainline Driving

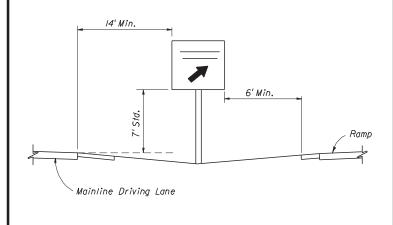
Lane

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For Use On All Rural, Freeway And Expressway Systems.

14' Min.

Ramp



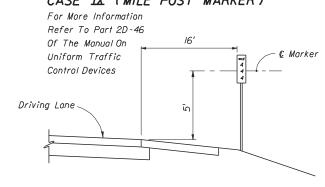
CASE VIII

Sign On Island

Driving Lane

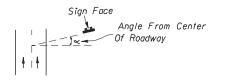
Center Sign Column On Island

CASE IX (MILE POST MARKER)



GENERAL NOTES:

- I. The typical sections shown hereon serve as a guide for locating the traffic signs required under various roadside conditions. For size and details of sign construction and footing, refer to the appropriate standard index drawing for roadside sign.
- 2. It shall be the CONTRACTORS responsibility to verify the length of sign supports in the field prior to fabrication.
- 3. Roadside signs shall be installed at an angle of I to 4 degrees away from the traffic flow (see illustration). Shoulder mounted signs shall be rotated counterclockwise and median mounted signs rotated clockwise. Signs on curves shall be mounted as noted above from the perpendicular to the motorist lign of sight.



- 4. The setback for stop and yield signs may be reduced to 3' minimum from the driving lane if required for visibility in business or residential sections with no curb and speeds of 30 MPH or less.
- 5. The mounting heights are measured from the bottom of the sign panel to a horizontal line extended from the edge of the driving lane. If the standard heights cannot be met, the minimum heights are as follows:

L	Expressway & Freeway Systems	/
(Other Roadway Systems	
1	Rural	5'
l	Jrban (including residential with	
1	parking and /or pedestrian activity)	7'

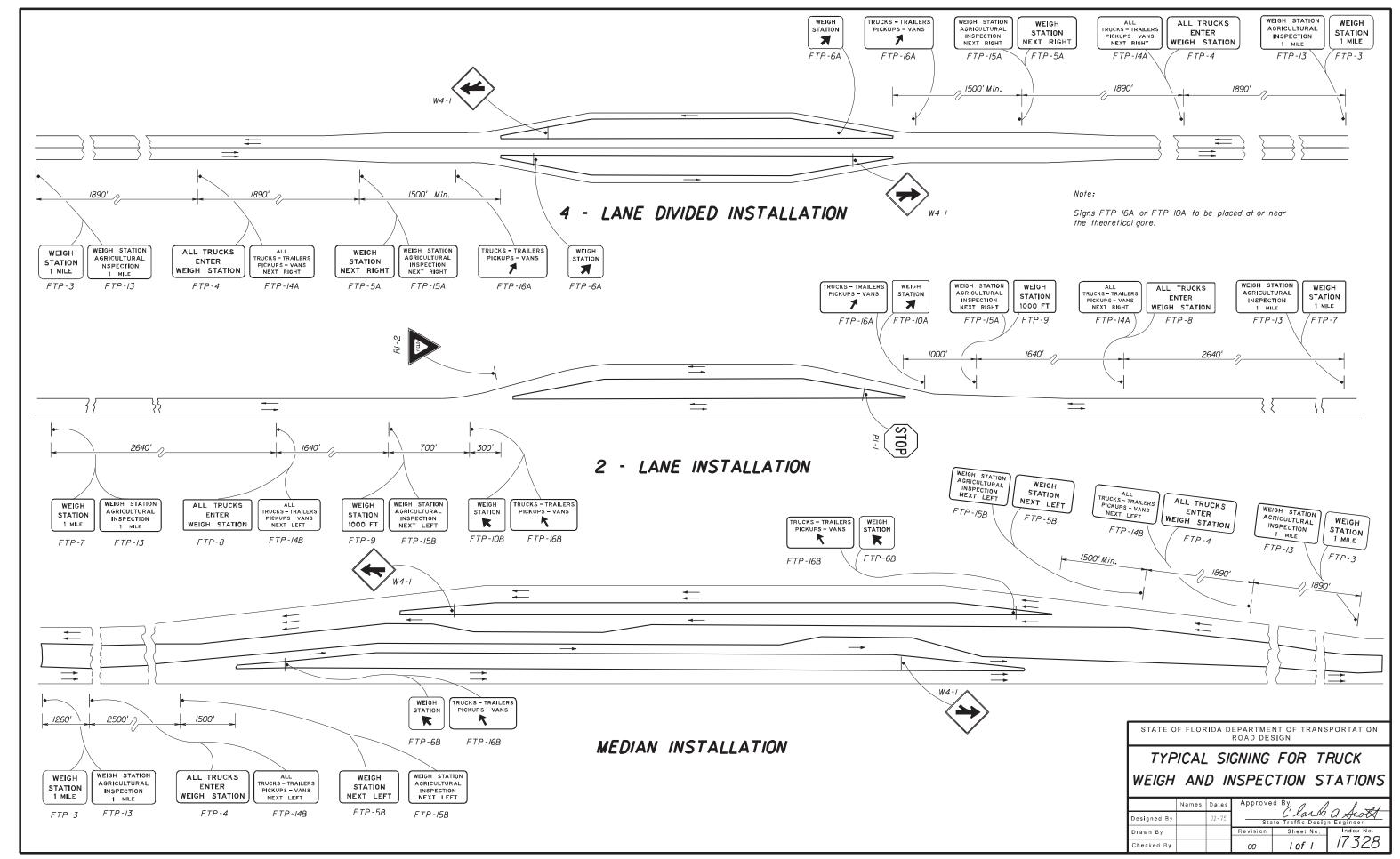
If a secondary sign is mounted below the major sign, the major sign shall be at least 8' and the secondary sign at least 5' for expressway & freeway systems and for other systems the height to the secondary sign shall be at least 4' for rural and 6' for urban sections.

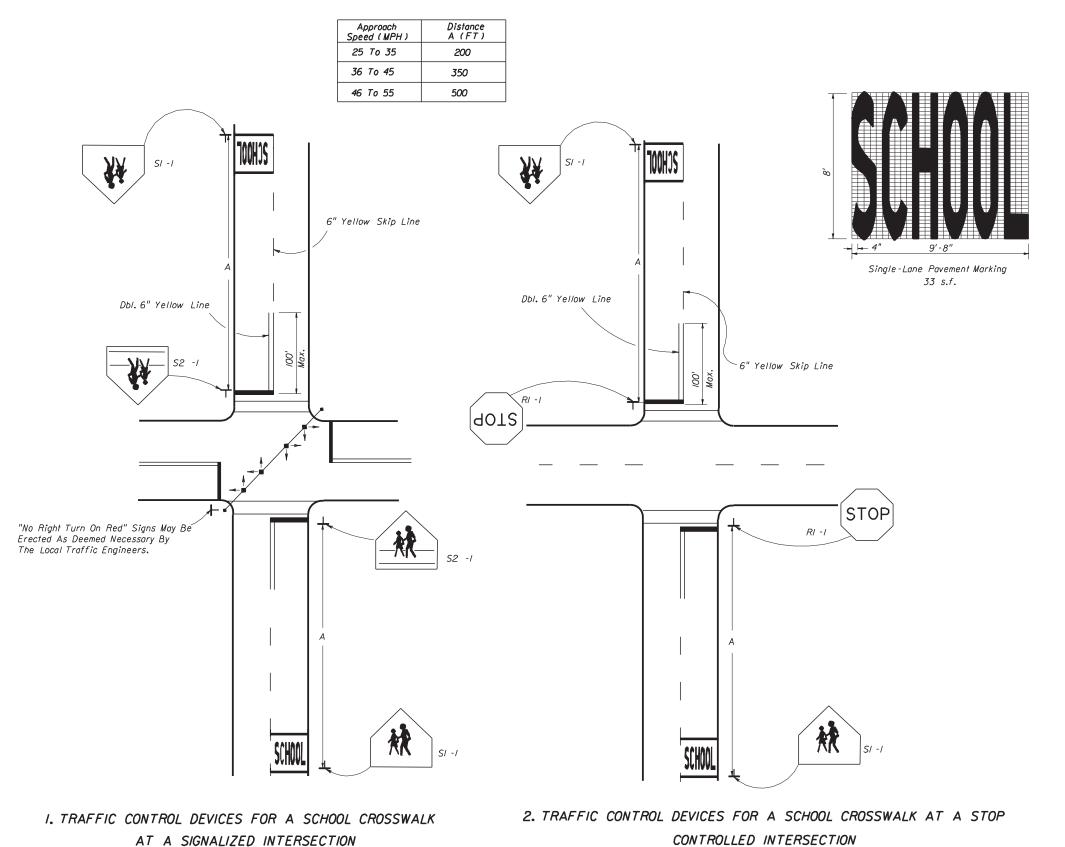
- Sign supports should never be placed in the bottom of ditches where erosion might affect the proper operation of the breakaway feature.
- 7. Sign supports shall not reduce the accessible route /continuous passage to less than 3'min. clear width as required by the Americans with Disabilities Act (ADA) Accessibility Guidelines.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

TYPICAL SECTIONS FOR PLACEMENT OF SINGLE & MULTI-COLUMN SIGNS

ı		Names	Dates	Approve		1 2 1 31					
ı	Designed By		3-75	Clark a Acold State Traffic Plans Engineer							
ı	Drawn By			Revision	Sheet No.	Index No.					
	Checked By			00	I of I	17302					



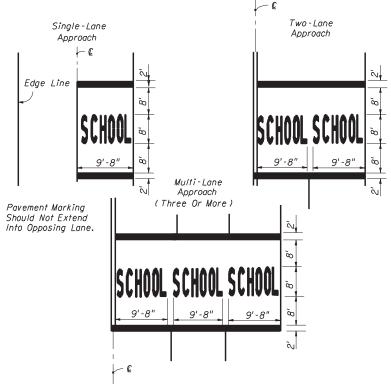


CONTROLLED INTERSECTION

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Special speed restrictions are not normally applicable to these two cases.



PAVEMENT MARKINGS

<u>Notes</u>

Signs shall be erected in accordance with Index No. 17302.

When computing pavement messages quantities do not include transverse lines.

All school signs shall be reflective.

School crosswalk width shall be 6' min. 10' std. without public sidewalk curb ramps 10' min. with public sidewwalk curb ramps. See Index No. 17346

For signalized intersections or mid-block signalized crossings where flashing beacon speed limit signs (post mounted or overhead) are installed, the minimum distance from the speed limit sign to the stop line shall be 100'. The sign shall not block the view of the signal.

> STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC PLANS

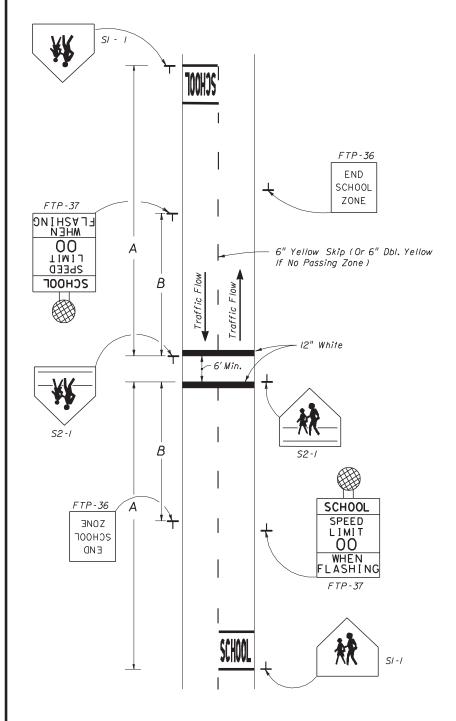
> SCHOOL SIGNS & MARKINGS

	Names	Dates	Approve		
Designed By		7-76	Sta	Clark ate Traffic Plans	
Drawn By			Revision	Sheet No.	Index No.
Checked By		7-76	00	I of 6	17344

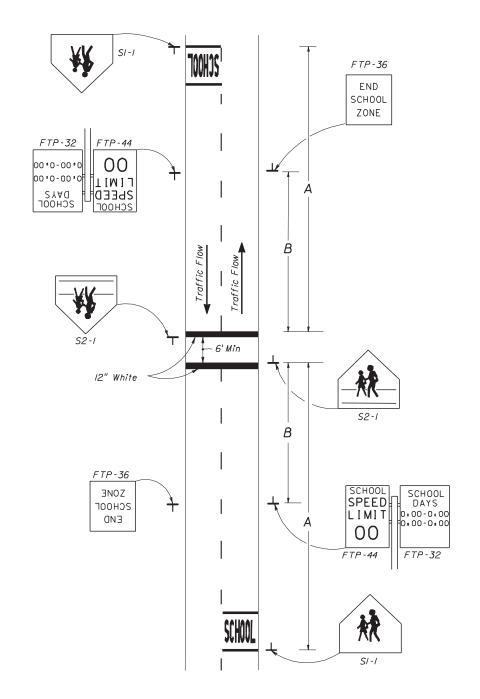
- 3. TRAFFIC CONTROL DEVICES WITH FLASHING BEACON FOR REDUCED SPEED ZONE AT A SCHOOL CROSSWALK

 (2 LANES 2 WAY TRAFFIC)

 (MIDBLOCK OR ON THRU STREET AT AN INTERSECTION)
- 4. TRAFFIC CONTROL DEVICES FOR A REDUCED SPEED ZONE AT A SCHOOL CROSSWALK (NO FLASHING BEACON)
 (2 LANES 2 WAY TRAFFIC)
 (MIDBLOCK OR ON THRU STREET AT AN INTERSECTION)



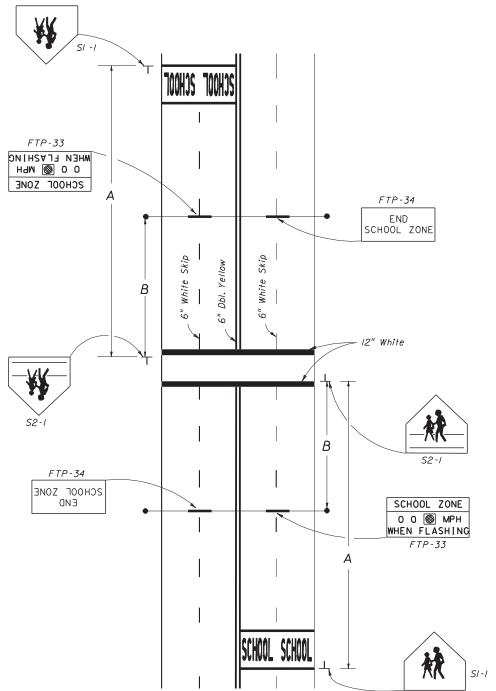
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APPROACH SPEED MPH	SUGGESTED DISTANCE IN FEET		
SI LLU MITTI	Α	В	
25 To 35	200	50	
36 To 45	350	65	
46 To 55	500	80	

5. TRAFFIC CONTROL DEVICES FOR A REDUCED SPEED ZONE AT A SCHOOL CROSSWALK WITH OVERHEAD FLASHING BEACON SPEED LIMIT SIGNS
(4 LANES UNDIVIDED - 2 WAY TRAFFIC)

(MIDBLOCK OR ON THRU STREET AT AN INTERSECTION)



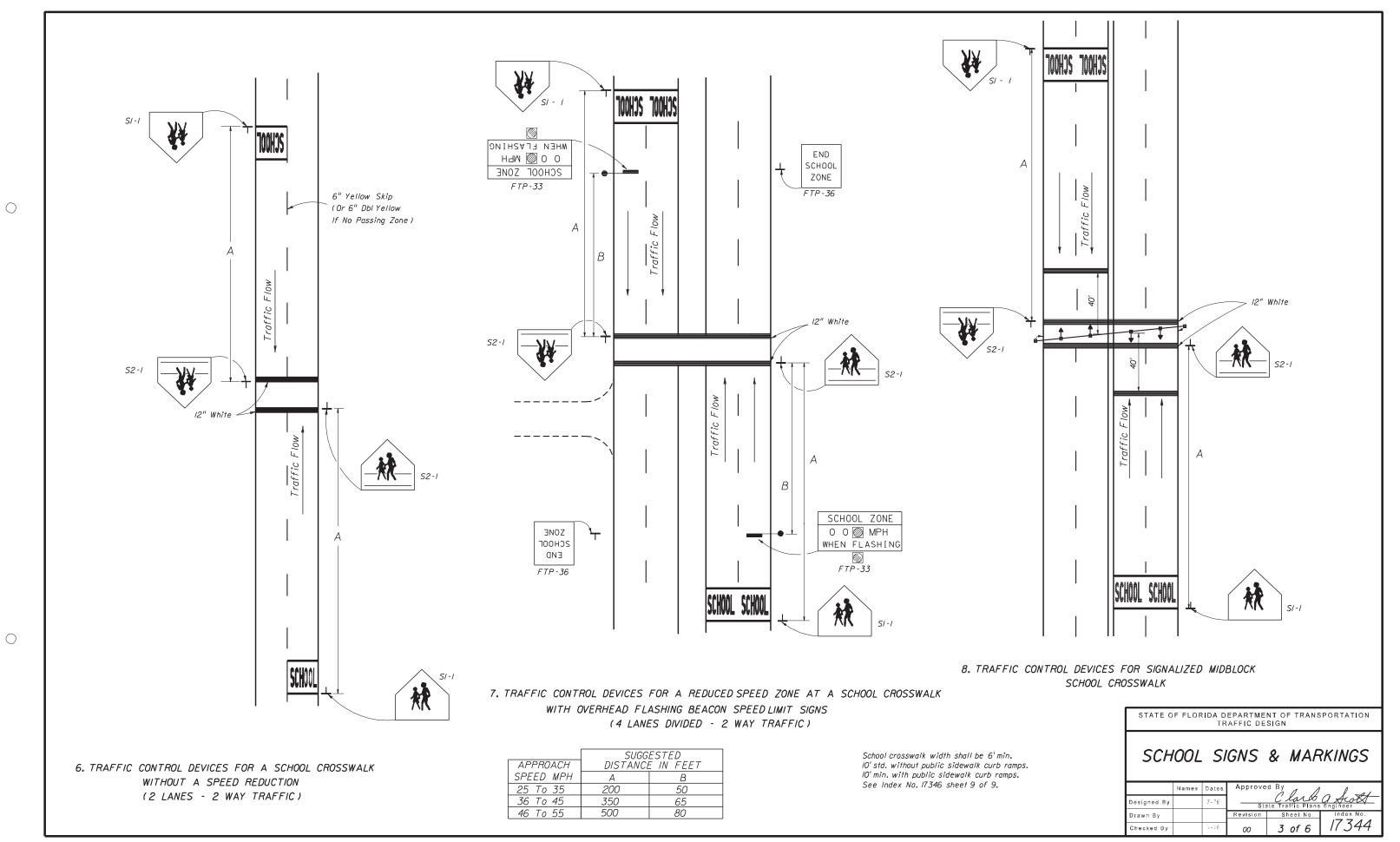
School crosswalk width shall be 6' min.

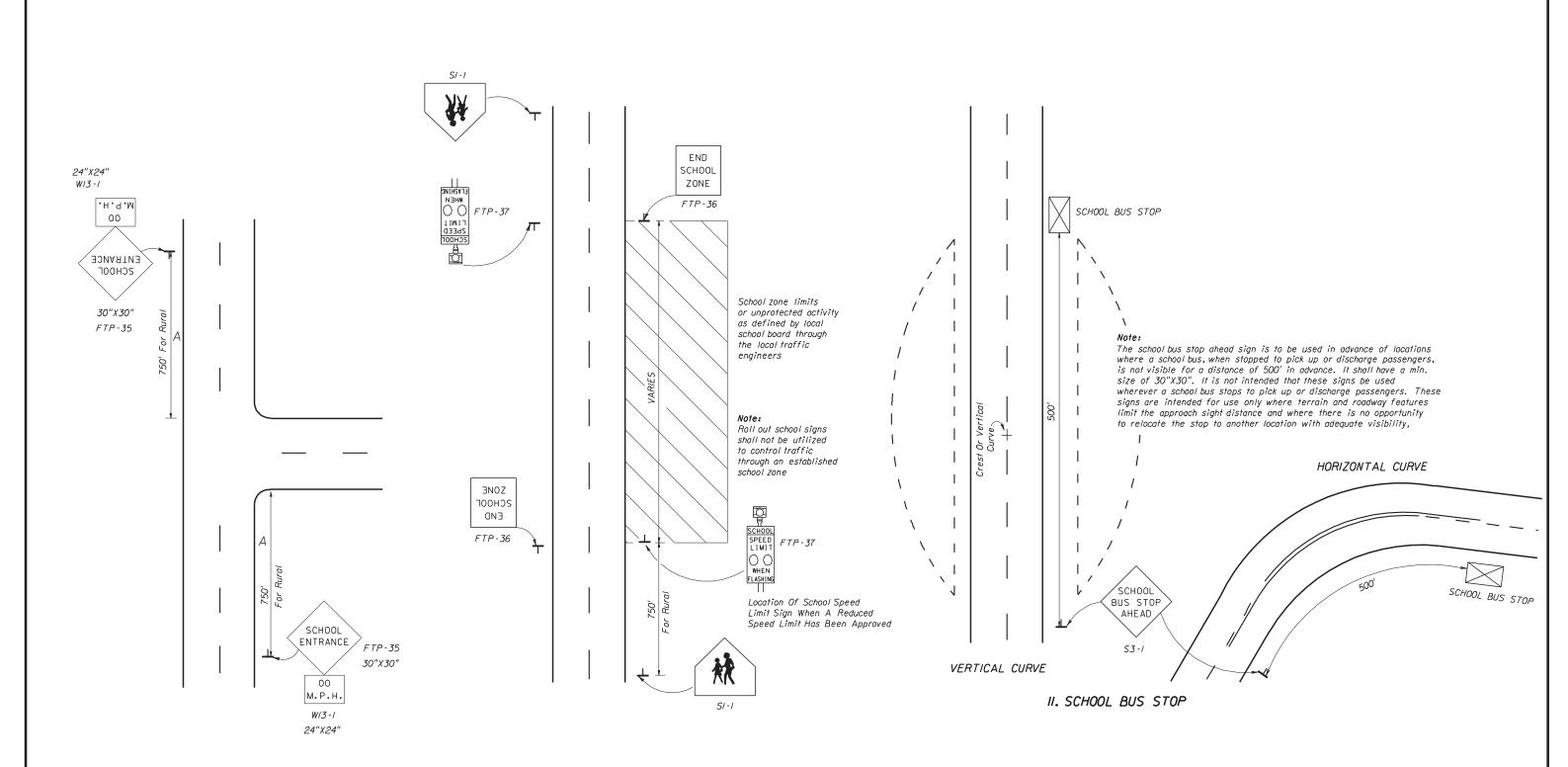
10' std. without public sidewalk curb ramps. 10' min. with public sidewalk curb ramps. See Index No. 17346 sheet 9 of 9.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

SCHOOL SIGNS & MARKINGS

	Names	Dates	Approve	d By	
Designed By		7-76	Sta	Clark ate Traffic Plans	
Drawn By			Revision	Sheet No.	Index No.
Checked By		7-76	00	2 of 6	17344





9. TRAFFIC CONTROL DEVICES AT SCHOOL ENTRANCES WITH LOW VOLUMES OF WALKING STUDENTS

These signs are intended for use only at those few locations where the school entrance is not evident to the motorist, and must be approved in advance by the responsible traffic engineering authority.

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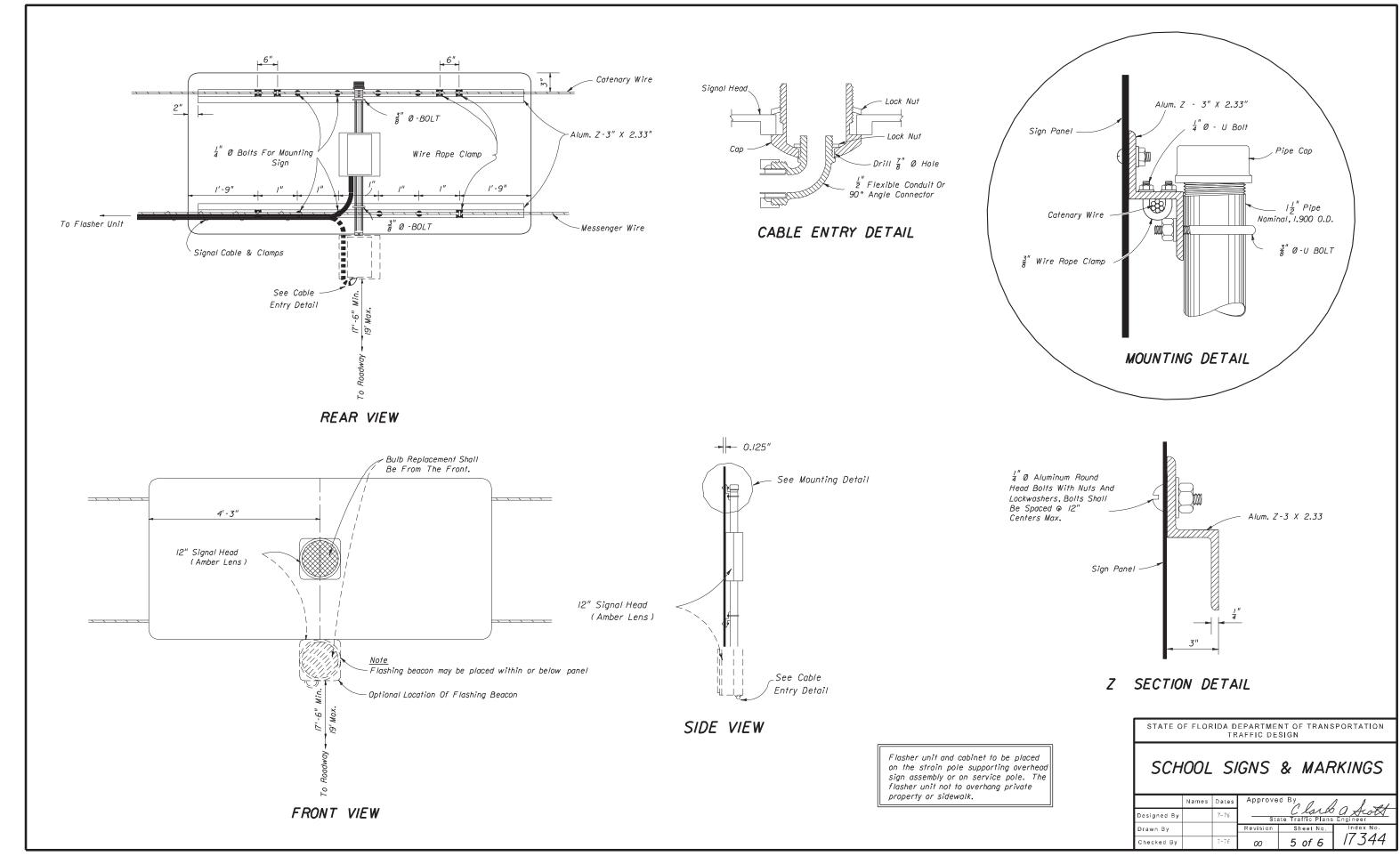
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IO. TRAFFIC CONTROL DEVICES FOR A TYPICAL SCHOOL ZONE FRONTING
THE SCHOOL PROPERTY

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

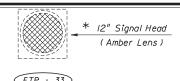
SCHOOL SIGNS & MARKINGS

	Names	Dates	Approve	20 11 11			
Designed By		7-76	C'larlo O Acold State Traffic Plans Engineer				
Drawn By			Revision	Sheet No.	Index No.		
Checked By		7-76	00	4 of 6	17344		



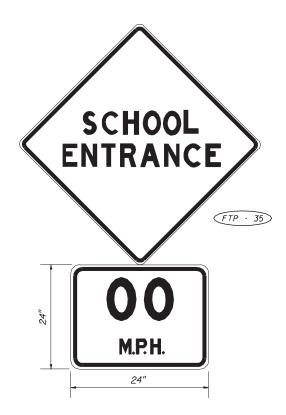
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SCHOOL ZONE WHEN FLASHING



OVERHEAD STANDARD

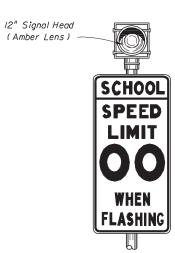
* Flashing Beacon May Be Placed Within Or Below Panel



W/3 - /

END SCHOOL ZONE





Ground Mount Standard

FTP - 37

Existing ground mount school speed limit signs utilizing a single 8" min. size beacon or two 6" min. size beacons inside the sign border are considered meeting the standard. However, replacement or upgrading of these school speed limit signs shall conform to the above standard. Numerical speed limit displayed shall be established by appropriate regulatory authorities.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

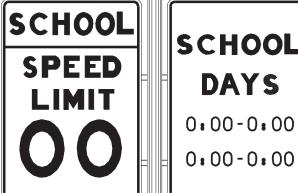
SCHOOL SIGNS & MARKINGS

ı		Names	Dates	Approve		1 1 21	
	Designed By		7-76	Clark O Acold State Traffic Plans Engineer			
ı	Drawn By			Revision	Sheet No.	Index No.	
	Checked By		7-76	00	6 of 6	17344	

Notes: SPEED LIMIT ASSEMBLY

- Standard size signs should be used whenever possible. Minimum sizes may be used only on low volume, low speed (less than 35 m.p.h.) streets. Special sizes should be used on expressway facilities where special emphasis is needed.
- 2. The value of the actual school zone speed limit shall be determined by the District Traffic Operations Engineer in cooperation with local school superintendents. In no case shall it be less than the 15 m.p.h. min. as set by law.
- 3. See Index No. 17355 for sign details.

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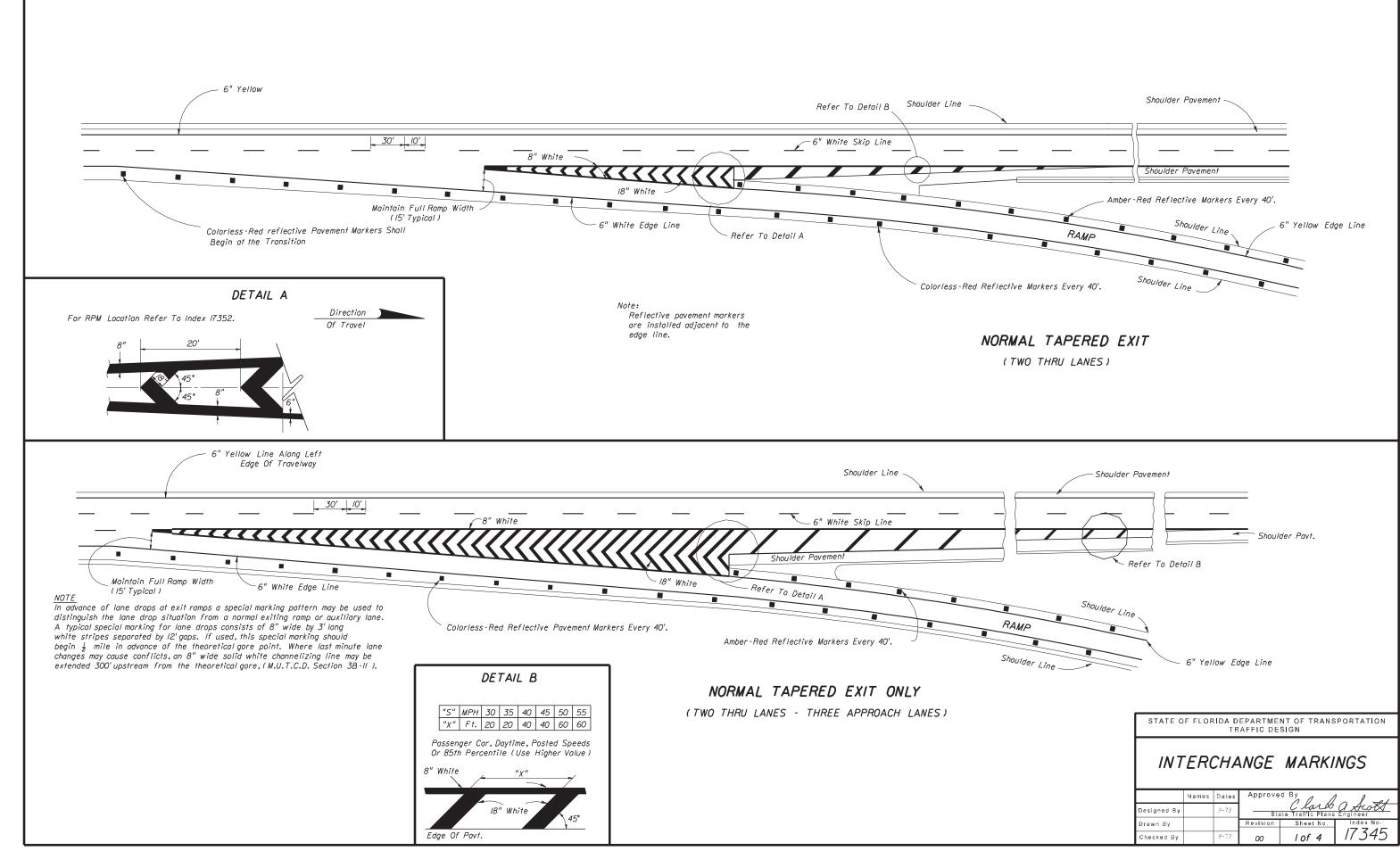
FTP-44

SCHOOL DAYS 0:00-0:00

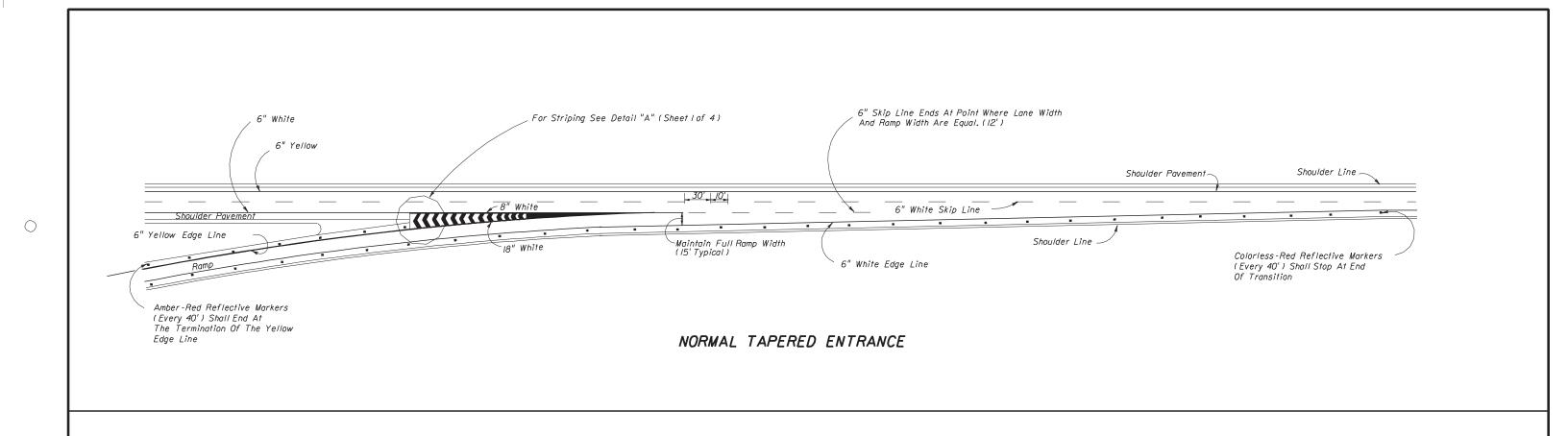
END SCHOOL ZONE

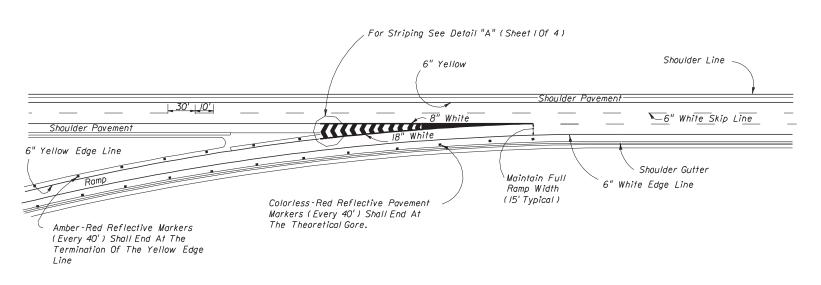
FTP - 36

(FTP - 32)



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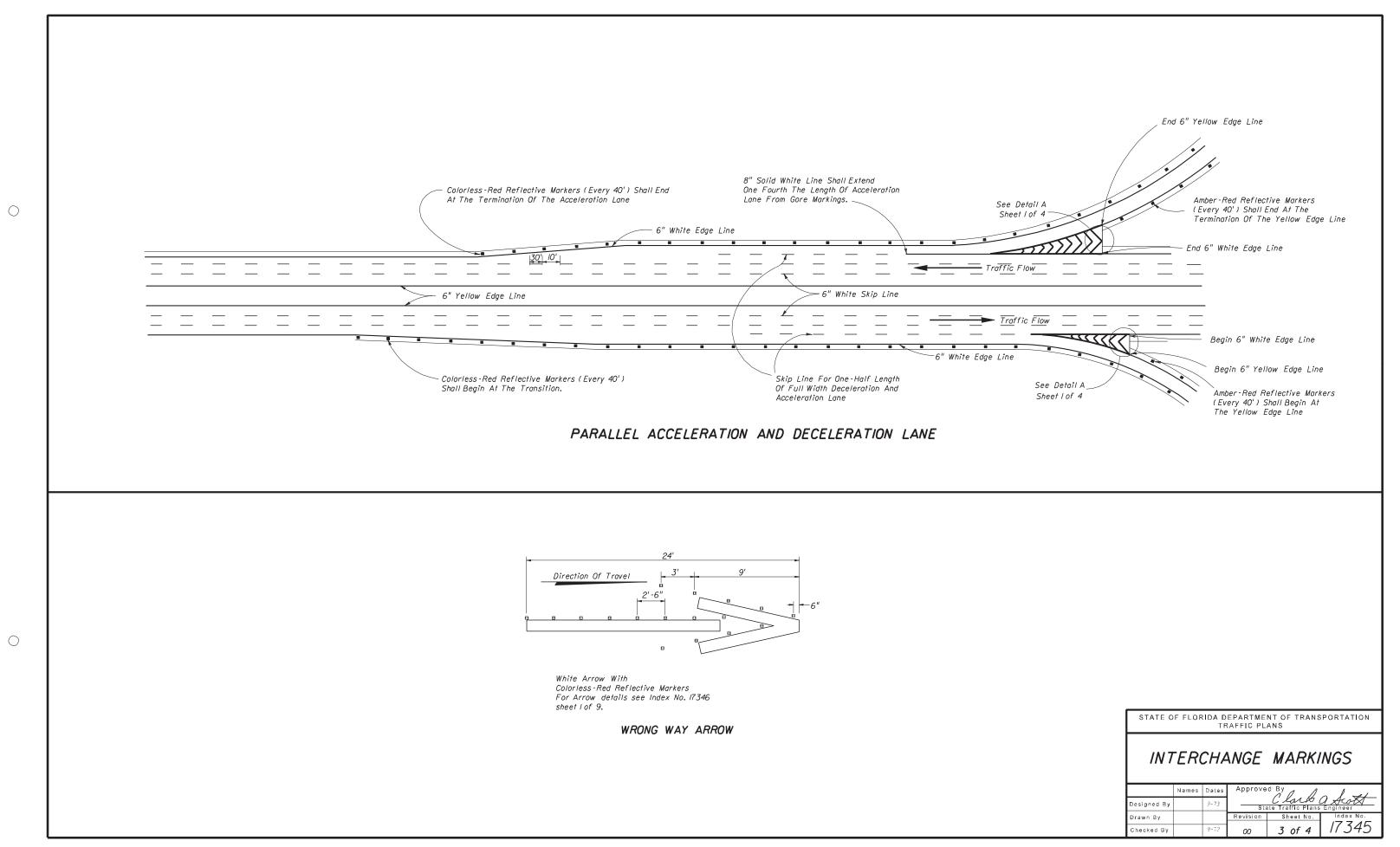


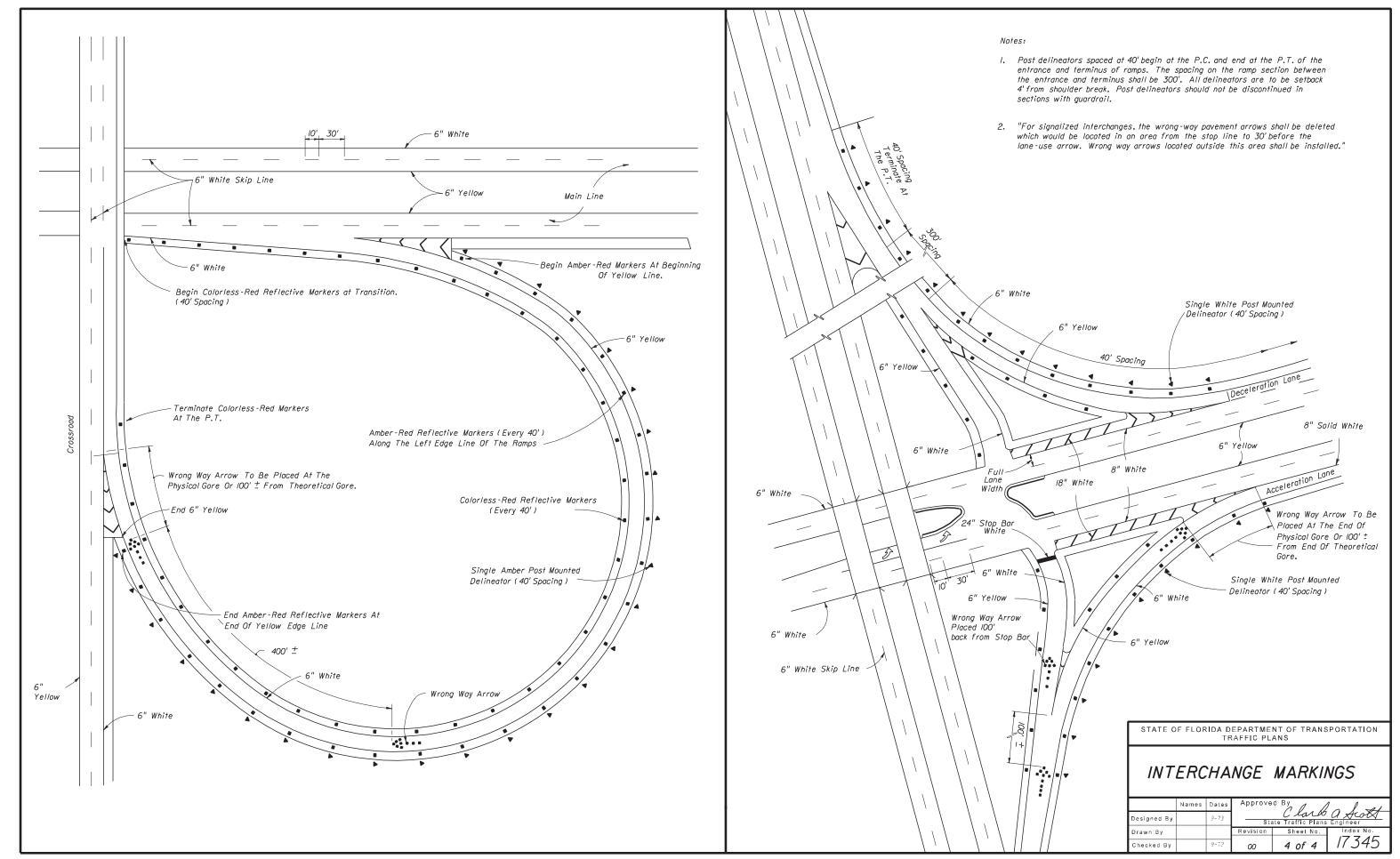
NORMAL TAPERED ENTRANCE
WITH ADDED LANE

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

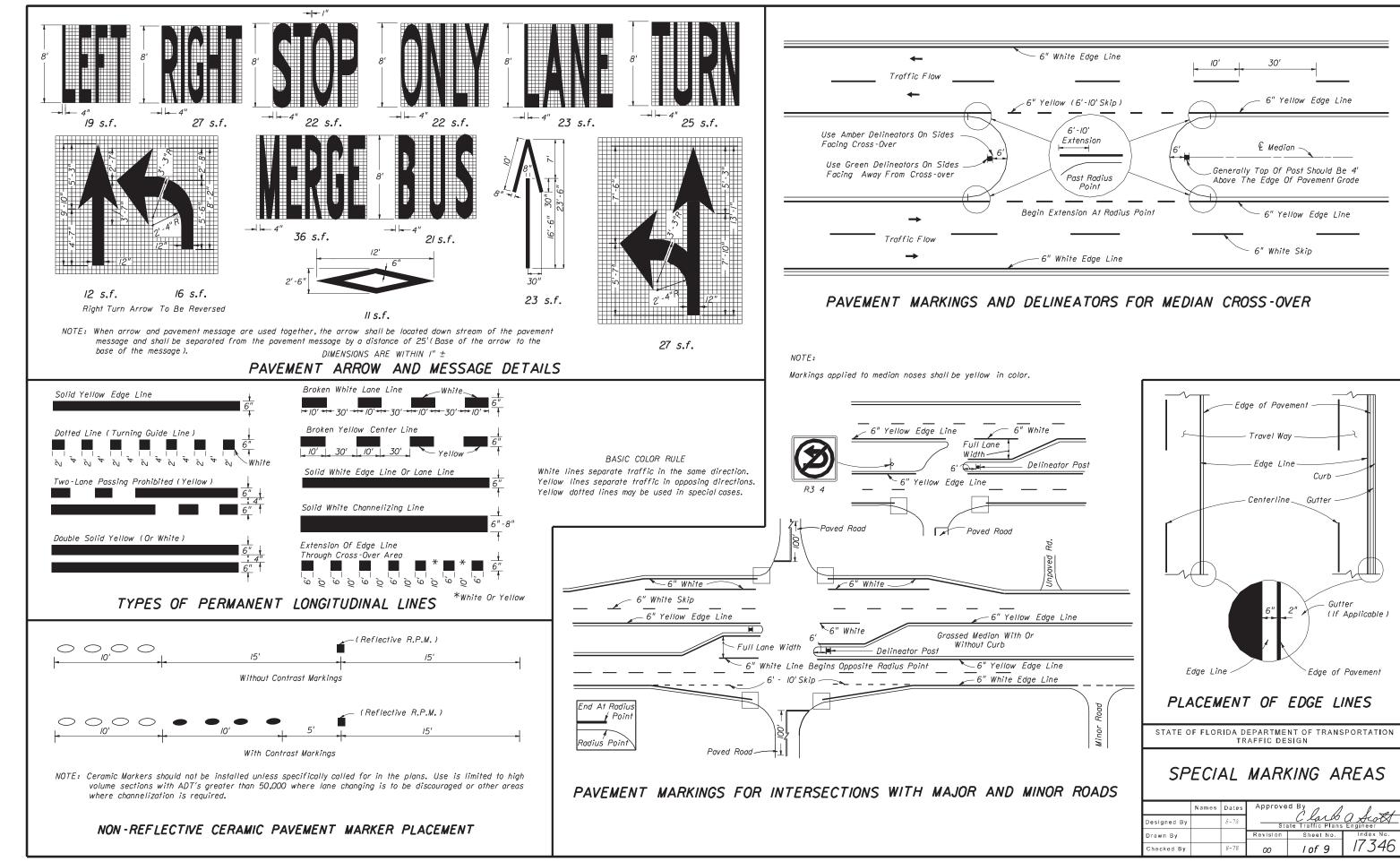
INTERCHANGE MARKINGS

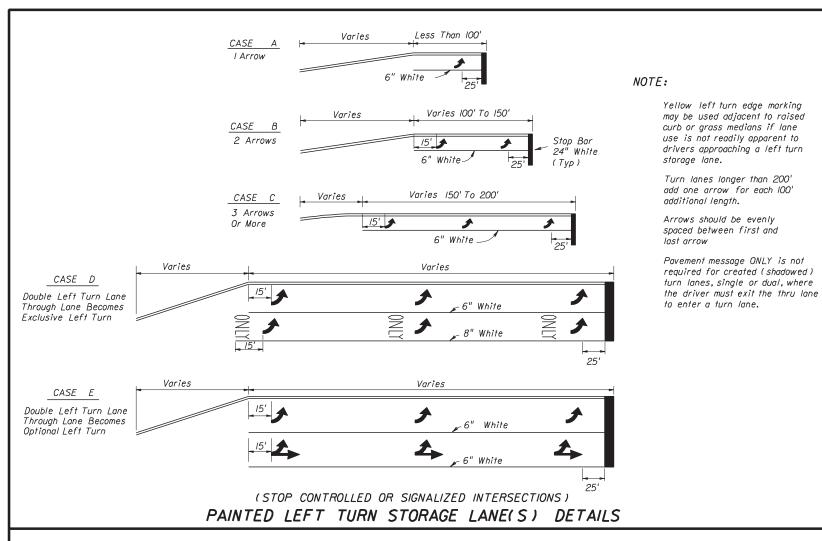
	Names	Dates	Approve	2 (1 (1	- 1 -1	
Designed By		7-73	Clark a Acott State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By		7-73	00	2 of 4	17345	



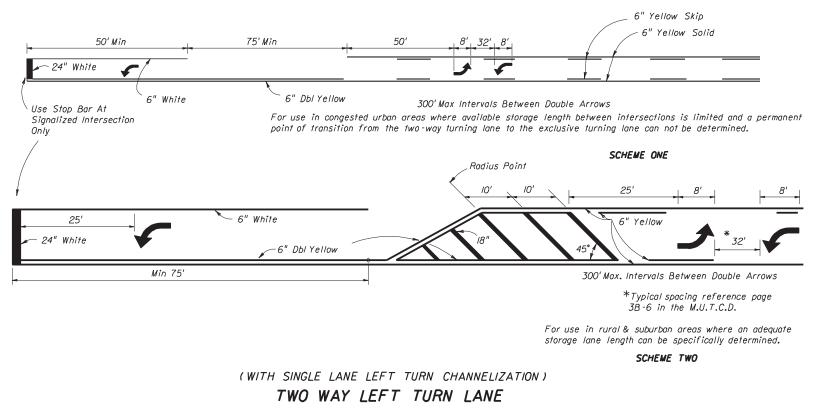


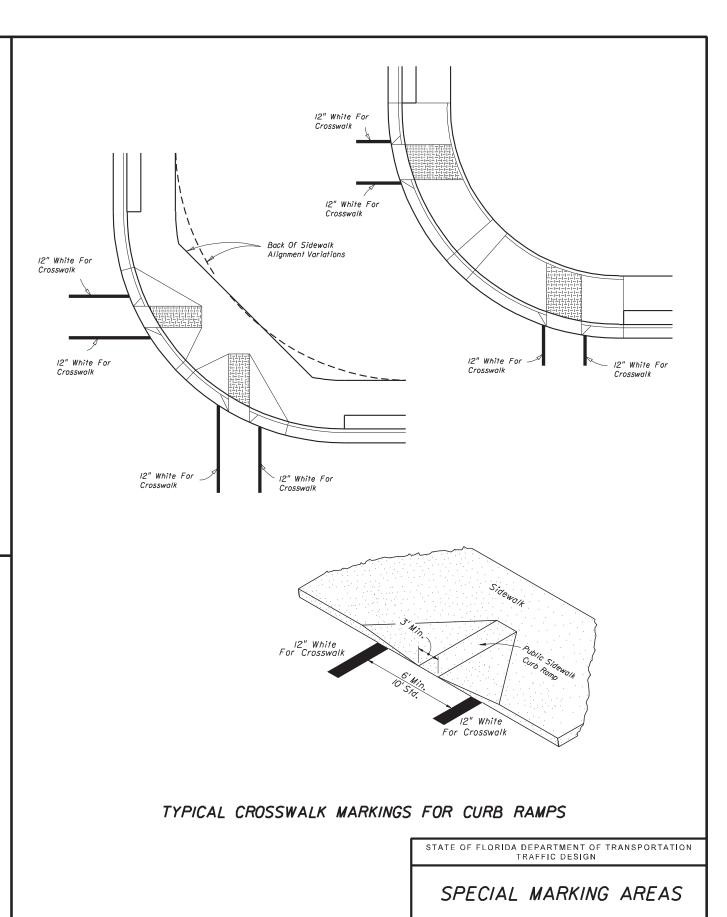
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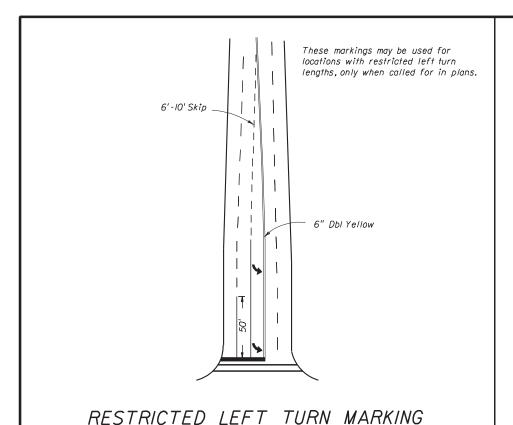
2 of 9

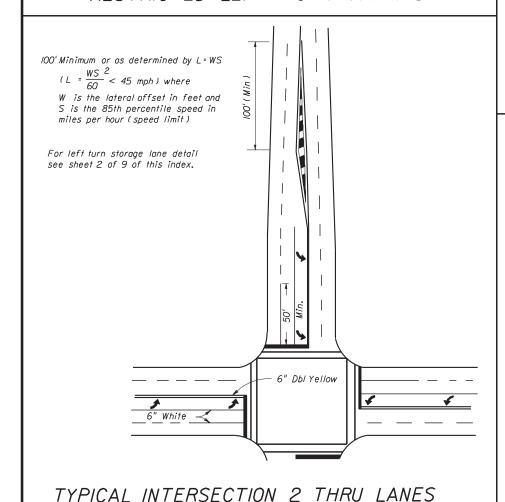
Names Dates

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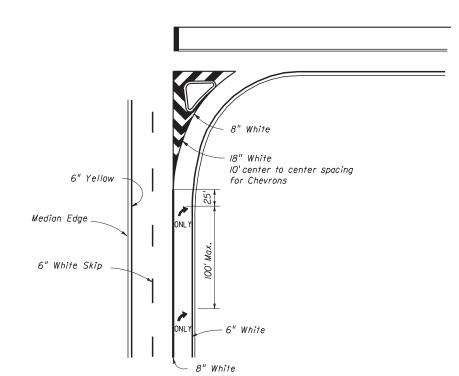
Designed By

rawn By Checked By

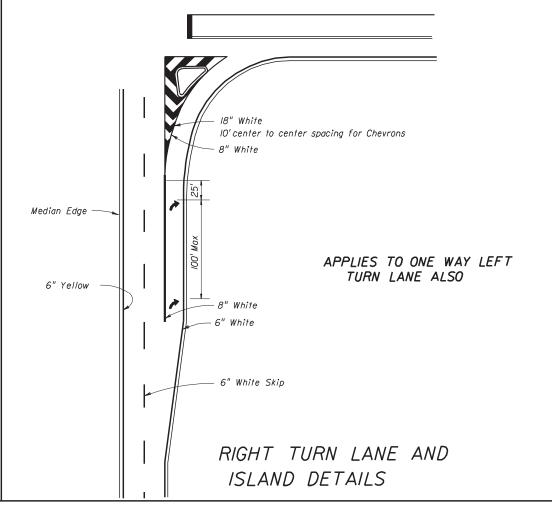


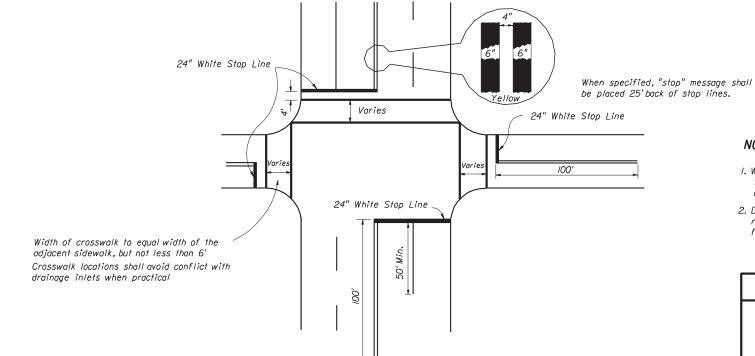


PLUS LEFT TURN LANE, WITH CROSSWALK



RIGHT TURN LANE DROP AND ISLAND DETAILS
LEFT TURN LANE DROP IS MIRROR IMAGE





STOP BARS, CROSSWALKS AND DOUBLE CENTER LINE DETAILS

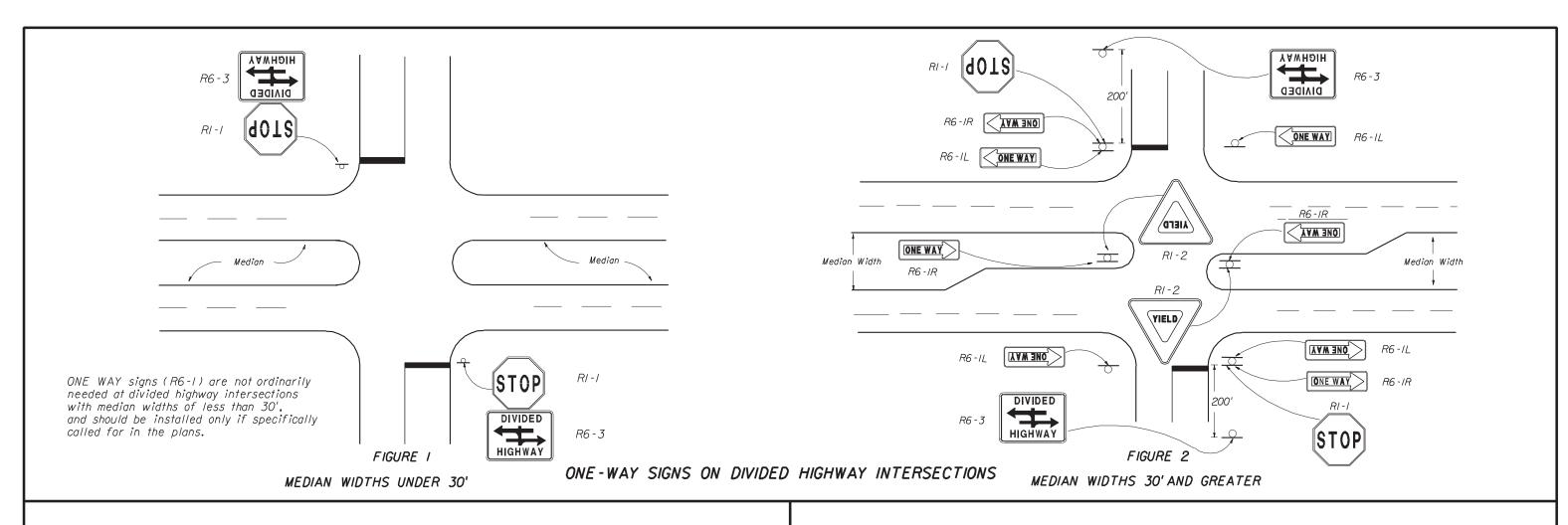
NOTES:

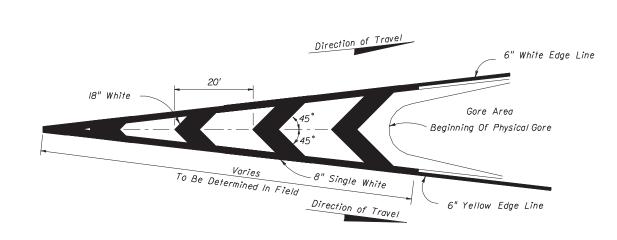
- I. When public sidewalk curb ramps are present, refer to sheet 2 of 9 & 7 of 9 of this index I7346 and Index No. 304 for crosswalk widths.
- Double yellow longitudinal center lines on all roadway approaches shall be extended back IOO' for projects involving intersection improvements only.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SPECIAL MARKING AREAS

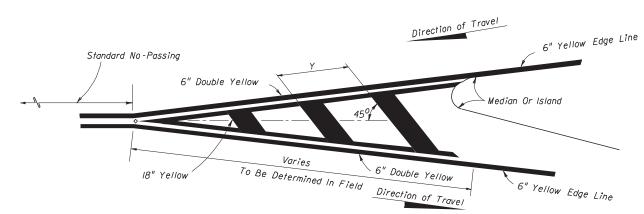
	Names	Dates	Approve		0 / 4/		
Designed By		9-76	Clark a Acold State Traffic Plans Engineer				
Drawn By			Revision	Sheet No.	Index No.		
Checked By		9-7€	00	3 of 9	17346		





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PAVEMENT MARKINGS FOR TRAFFIC CHANNELIZATION AT GORE
(TRAFFIC FLOWS IN SAME DIRECTION)



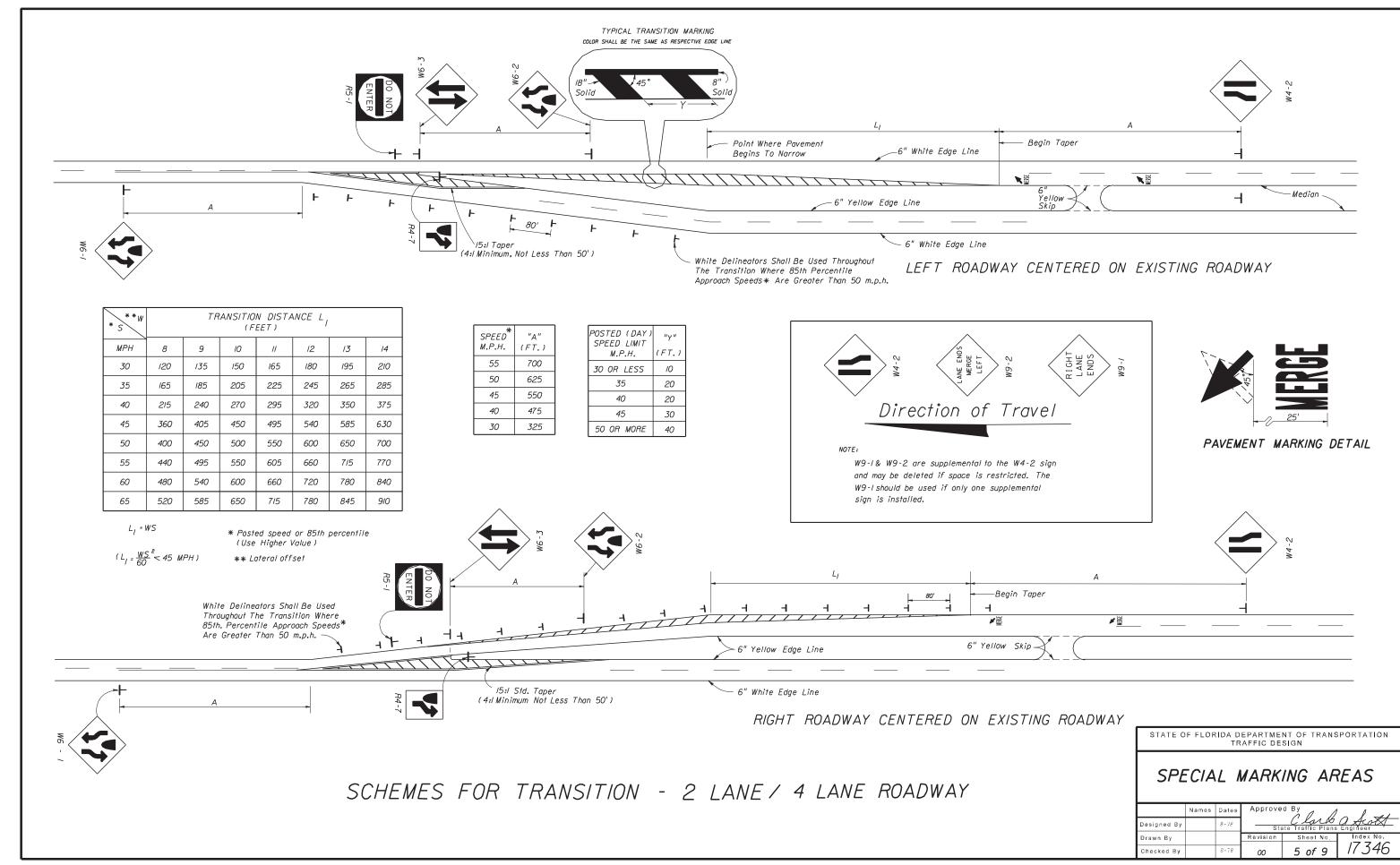
PAVEMENT MARKING FOR TRAFFIC SEPARATION (TRAFFIC FLOWS IN OPPOSING DIRECTIONS)

POSTED (DAY) SPEED LIMIT M.P.H.	"Y" ft
30 OR LESS	10
35	20
40	20
45	30
50 OR MORE	40

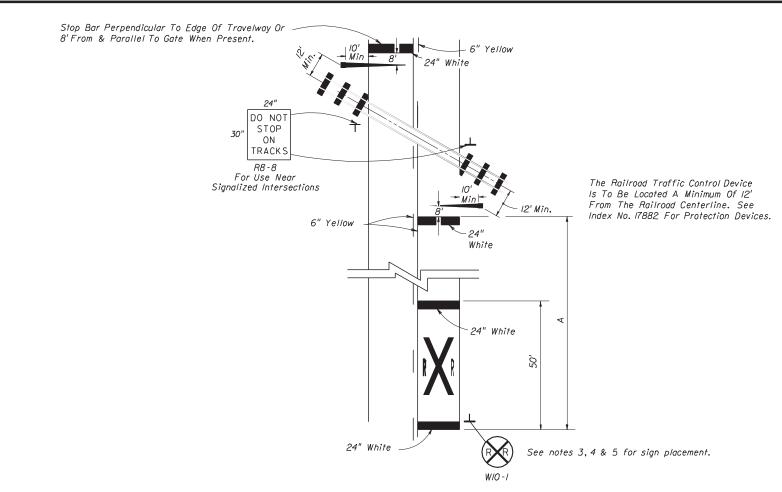
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

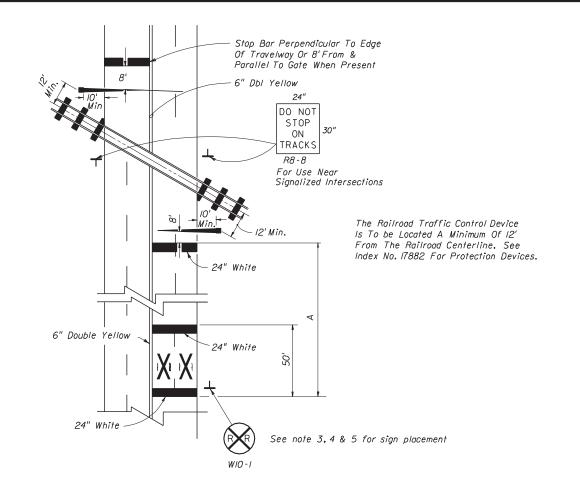
SPECIAL MARKING AREAS

	Names	Dates	Approve		1 - 1 -11	
Designed By		8-78	Clark a Acolf State Traffic Plans Enginéer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By		8-78	00	4 of 9	17346	



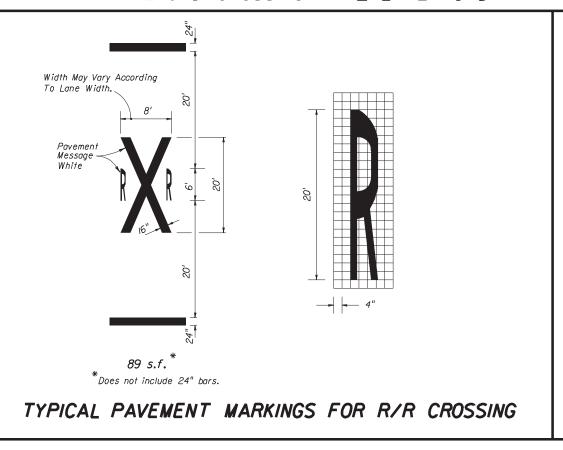
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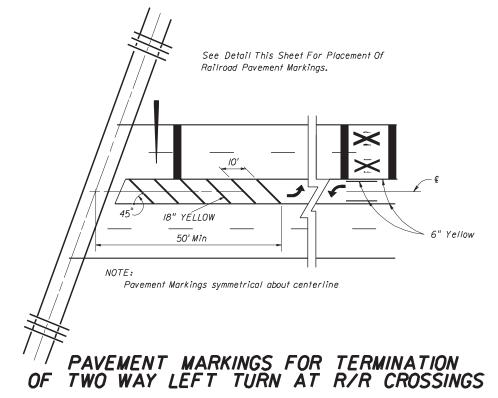




RAILROAD CROSSING AT 2-LANE ROADWAY

RAILROAD CROSSING AT 4-LANE ROADWAY





NOTES:

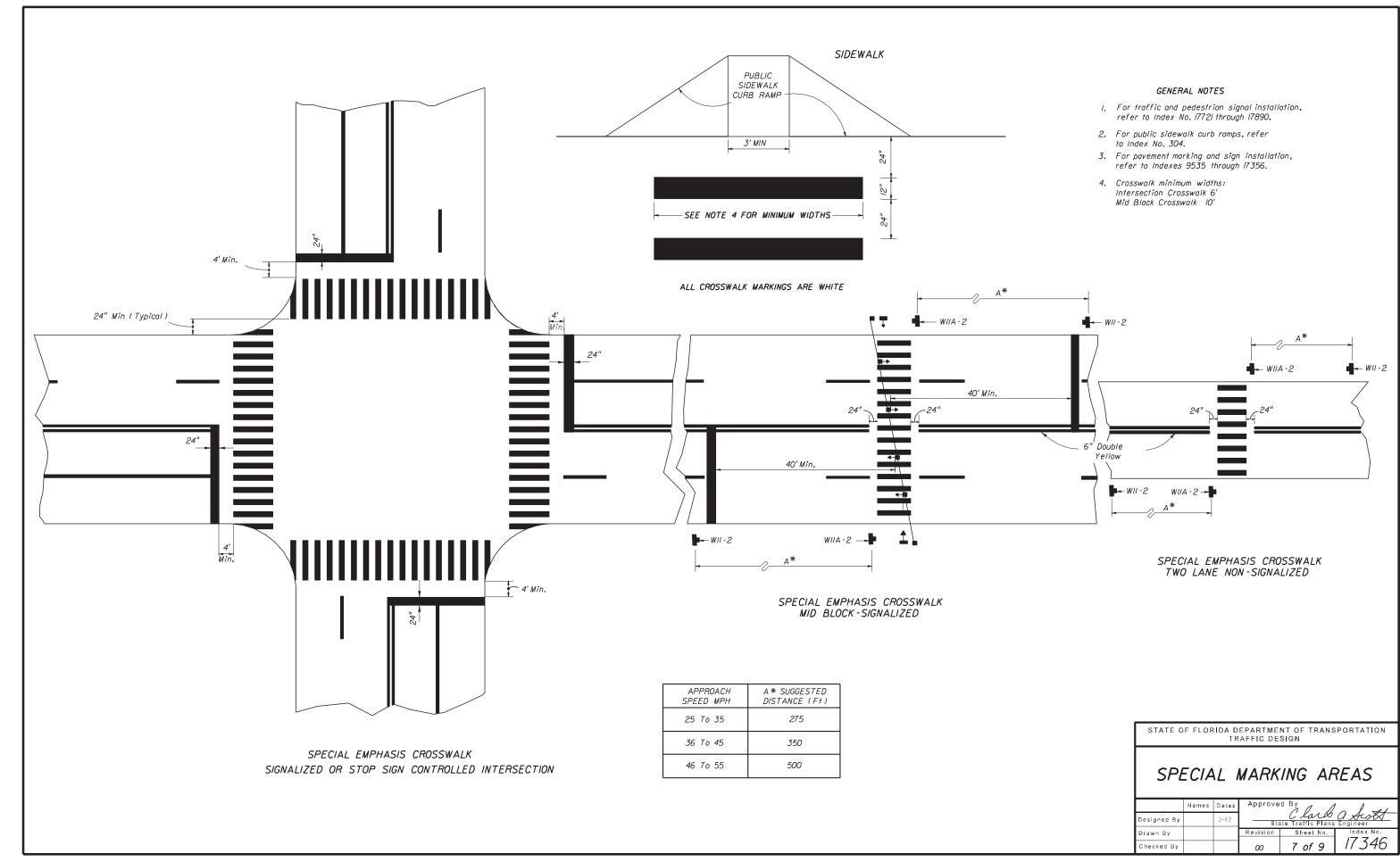
- When computing pavement messages, quantities do not include transverse lines.
- When dynamic devices are not present or are to be installed, the crossbuck shall be located at the future location of the RR gate or signal and gate in accordance with Index No. 17882.
- 3. Placement of sign WIO-I in a residential or business district, where low speeds are prevalent, the WIO-I sign may be placed a minimum distance of IOO' from the crossing. Where street intersections occur between the RR pavement message and the tracks an additional WIO-I sign & additional pavement message should be used.
- 4. Recommended location for FTP-38 or FTP 38B sign, l00' urban & 300' rural in advanced of the crossing.
- 5. A portion of the pavement marking symbol should be directly opposite the WIO-1 sign.

SPEED MPH	A (<i>Ft</i>)
65	650
60	550
55	450
50	375
45	300
40	225
35	150
30	100
Urban	50 Min.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

SPECIAL MARKING AREAS

	Name	s Dates	Approve	<i>à []</i>	- 1 -11		
Designed	Ву	6-76	Clark of Acold State Traffic Plans Engineer				
Drawn By			Revision	Sheet No.	Index No.		
Checked 6	Зу	6-76	00	6 of 9	17346		



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Markings in or adjacent to bike lanes should be thermoplastic with a mixture of 50 percent glass spheres and 50 percent sharp silica sand applied at a rate of 0.2 lb/ft². The sharp silica sand shall meet the following gradation requirements: Sieve Size Percent Passing (µ, m) 850 100 300 0-10 (Florida Standard Spec. 711-4.6) Recommended spacing of diamond symbol: Immediately after intersections and major driveways and at a maximum spacing of 600 feet for urban sections and 1320 feet for rural Raised pavement markings and raised barriers can cause steering difficulties and should not be used to delineate bicycle lanes. All pavement markings and pavement messages shall be white.

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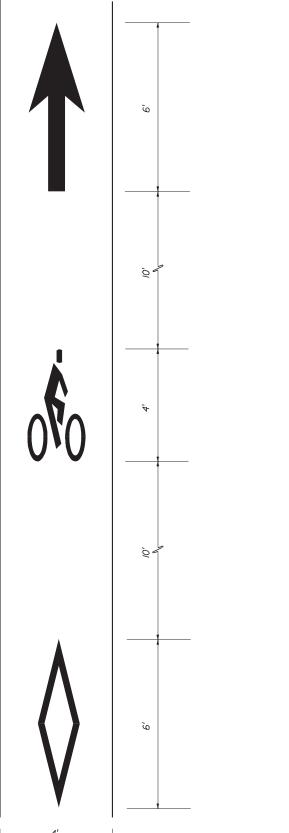
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Number

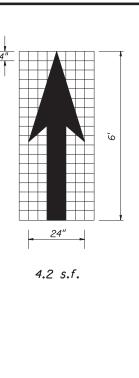
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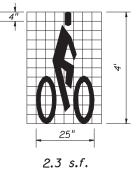
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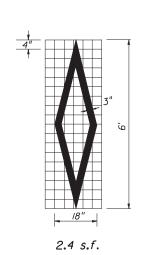
sections.

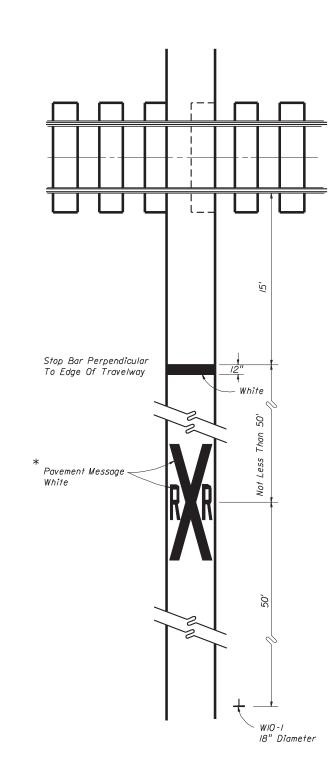


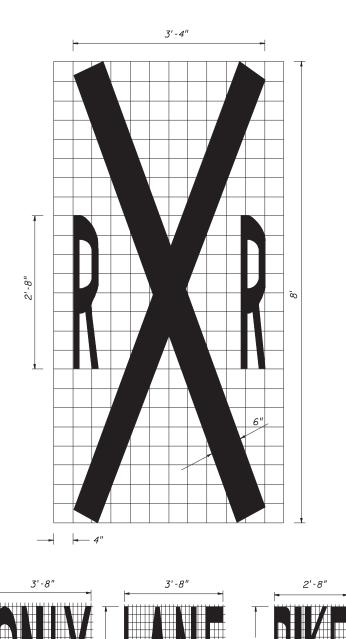
DETAIL OF BIKE LANE MARKINGS











PAVEMENT MESSAGE DETAILS

// s.f.

II s.f.

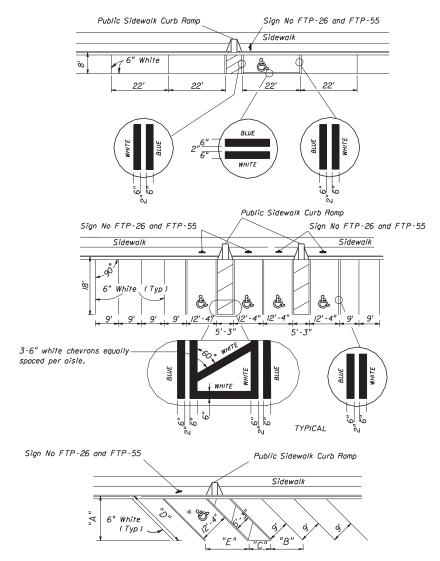
When used on a bike lane (adjacent to vehicle lane) markings shall be placed adjacent to markings for vehicles & WIO-I sign shall be sized and placed for vehicles.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

SPECIAL MARKING AREAS (BICYCLE)

	Names	Dates	Approve		
Designed By		8-84	Sta	Clark ate Traffic Plans	C Acold Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	8 of 9	17346

11.3 s.f.



* FOR ACCESSIBLE MARKINGS - SEE ABOVE

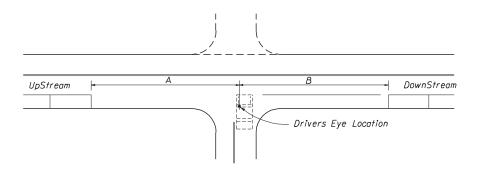
	"DIMENSIONS"								
Δ°	"A"	"B"	"C"	"D"	"E"				
45°	19' -1"	12'-9"	7'-6"	27'-0"	17'-8"				
60°	20'-1"	10'-5"	6'-4"	23'-2"	14' - 4"				

NOTES.

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- I. Dimensions are to the centerline of markings.
- 2. An Access Aisle is required for each accessible space when angle parking is used.
- 3. Criteria for pavement markings only, not public sidewalk curb ramp locations. For ramp locations refer to plans.
- 4. Blue pavement markings shall be tinted to match shade I5180 of Federal Standards 595a.
- 5. The FTP-55 panel shall be mounted below the FTP-26 sign.

PAVEMENT MARKING FOR PUBLIC SIDEWALK CURB RAMPS IN REST AREAS

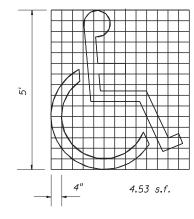


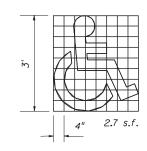
SPEED	UP STREAM (A)	DOWN STA	REAM (B)
MPH		2 LANE	4 LANE
0-30	85'	60'	<i>4</i> 5′
35	100'	70'	50'

NOTES

- Distances measured longitudinally along the street from driver location of entering vehicle to end of parking restriction.
- 2. Distances applicable to intersecting street, major driveways and other driveways to the extent practical.
- 3. For non-signalized intersections, the values above shall be compared with the values for signalized intersections and the maximum restrictions implemented. These restrictions apply to both accessible and nonaccessible parking.

MINIMUM PARKING RESTRICTION FOR NON-SIGNALIZED INTERSECTIONS





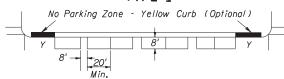
Use of pavement symbol in accessible parking spaces is optional, when used the symbol shall be 3' or 5' high and white in color.

"UNIVERSAL SYMBOL OF ACCESSIBILITY"

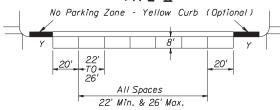
GENERAL NOTES (Signalized & Non-signalized)

- I. For entrances to a one-way street, the downstream restriction may be reduced to 20'.
- 2. Parking shall not be allowed within 20' of a crosswalk.
- 3. All parking lane markings shall be 6" white.
- 4. Parking lane lines shall be broken at driveways.
- 5. Refer to Chapter 316, Fla. statutes, for laws governing parking spaces.
- 6. Where curb and gutter is used, the gutter pan width may be included as part of the minimum width of parking lane, but desirably the lane width should be in addition to that of the gutter pan.

TYPE I



TYPE II



TYPE III



SPEED LIMIT MPH	SIGNALIZED INTERSECTIONS	
0 - 30	30	DISTANCE FROM CURB RADIUS (Y)
35	50	

PARKING RESTRICTION (FT.) FOR SIGNALIZED INTERSECTION

NOTES:

- I. Parking restrictions measured from curb radius point.
- 2. Restrictions for accessible parking are the same as those applied to non-signalized intersections.

MINIMUM PARKING RESTRICTION FOR SIGNALIZED INTERSECTION

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

SPECIAL MARKING AREAS (PARKING)

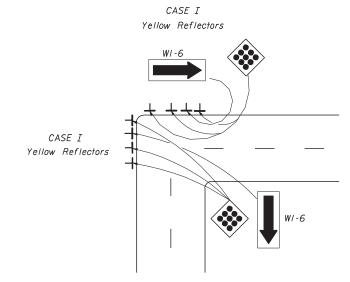
	Names	Dates	Approve	d By	101 41
Designed By		8-86	Clark O Acold State Traffic Plans Engineer		
Drawn By			Revision	Sheet No.	Index No.
Checked By		8-8€	00	9 of 9	17346

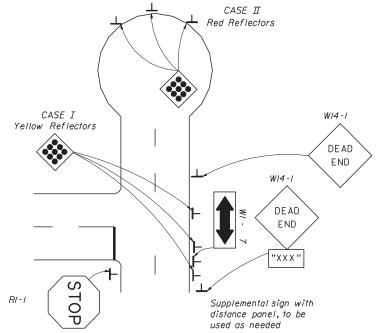
- CASE I Type I Object Markers shall consist of nine yellow reflectors mounted on a yellow reflective background or consist of a reflective panel of the same size with Type III-A, III-B or III-C yellow sheeting.
- CASE II End of Road Markers shall consist of nine red reflectors mounted on a red reflective background or consist of a reflective panel of the same size with Type III-A, III-B or III-C red sheeting.

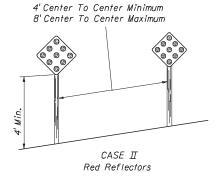
NOTES:

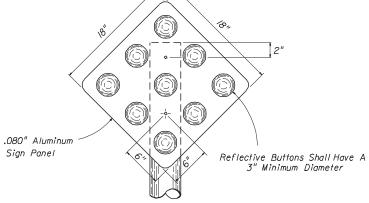
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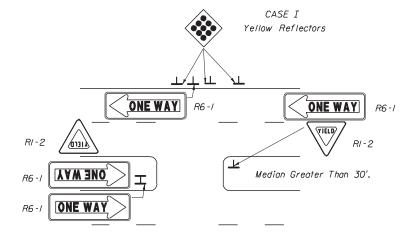
- This index applicable to residential and minor streets only. Major streets to be evaluated on a case by case basis.
- 2. "T"-intersection-Two-Way arrows and reflectors are optional. The need should be based on a review of each location.
- 3. For additional details on aluminum round post, steel flanged channel post, sign panel material and bolts, nuts and washers see Index Nos. II860 and II865.
- 4. Case I Installation The arrow panels and object markers shall be located approximately 20', but not less than 12' from the edge of the travel lane.
- 5. Dead end sign shall be posted a sufficient advance distance to permit the vehicle operator to avoid the dead end by turning off, if possible, at the nearest intersecting street.
- 6. For pavement marking see index no. 17346
- 7. No guardrail is required unless special field conditions require its use.

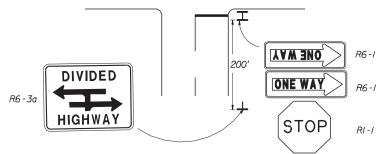


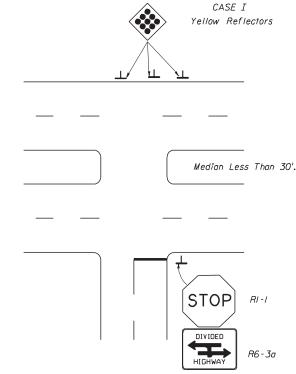












ONE WAY signs (R6-I) are not ordinarily needed at divided highway intersections with median widths of less than 30', and should be installed only if specifically called for in the plans.

Supports shall be driven 3' into the ground.

2" Ø X $\frac{1}{8}$ " Aluminum Round Post or 2.5 $^{\#}/Ft$. Steel Flanged Channel Post.

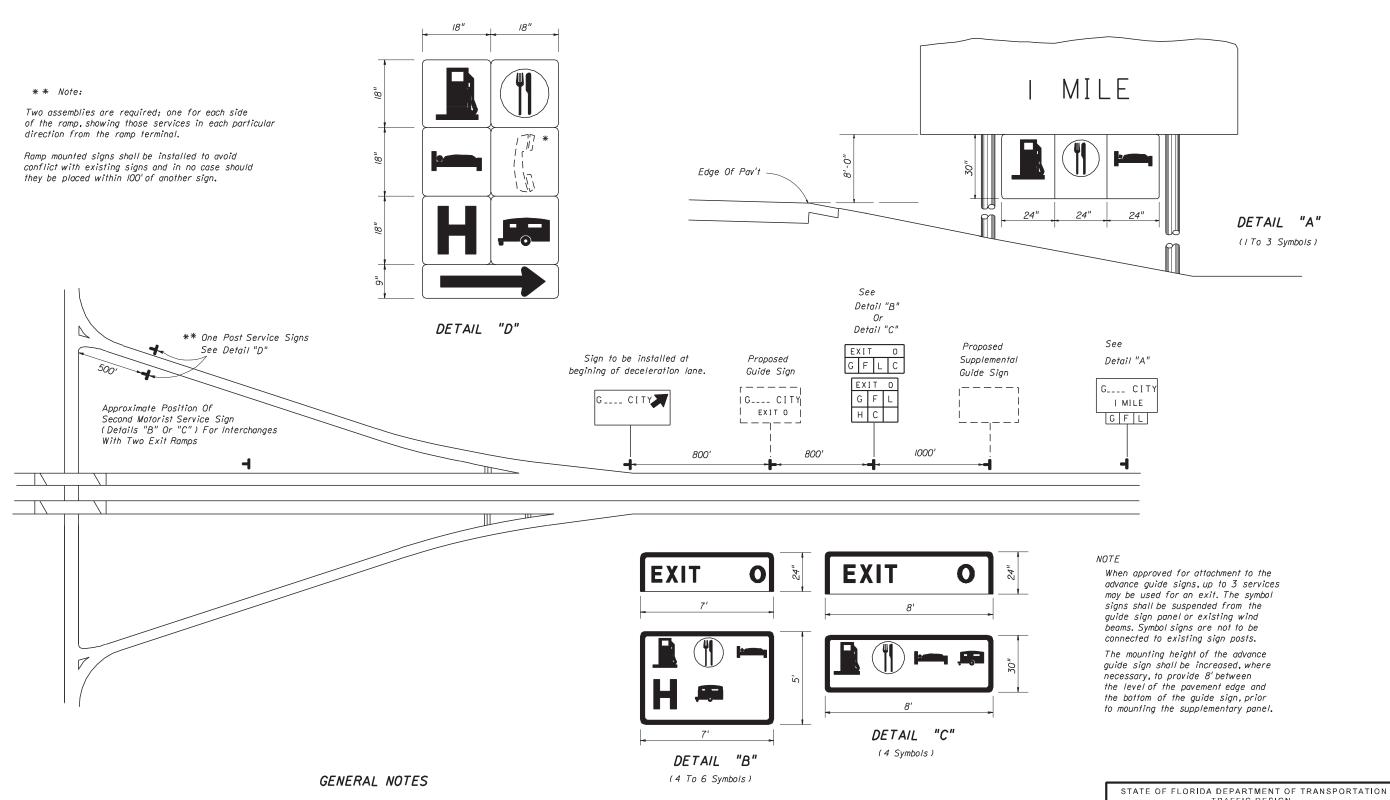
Aluminum Post: $\frac{\pi}{8}$ 0 Aluminum Button Head Bolt with Nut and Lockwasher or $\frac{15}{16}$ 0 Stainless Steel Hex Head Bolt with Flat Washer under Head and Lockwasher under Nut.

Channel Post: Provide Attachment in Accordance with the "Sign Attachment Detail" on Index No. I/865.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

TRAFFIC CONTROLS FOR STREET TERMINATIONS

	Names	Dates	Approve	1. 11 11	2 1 AL
Designed By		11 74	State Traffic Plans Engineer		
Drawn By			Revision	Sheet No.	Index No.
Checked By		11 74	00	I of I	17349



- I Only those services meeting criteria established by the Department and approved by the State Traffic Operations Engineer for each interchange shall be shown. Symbol signs for motorist services shall always appear in the following order reading from left to right and top to bottom: Gas, Food, Lodging, Phone * , Hospital, Camping.
- * The phone symbol shall not be shown whenever any Gas, Food, Lodging or Camping symbol appears.
- 2 Symbols shall appear consecutively on the sign with no positions left blank or reserved for intermediate symbols not currently approved for a particular interchange.

- 3 All motorist service signs to have White Legend and Border with Blue Background.
- 4 For mounting details see Index 9535 for Type "A" breakaway or Index 11860 for Type "C" Frangibility.

SIGNING FOR MOTORIST **SERVICES**

	Names	Dates						
esigned By		3-76	C'Lack O Acold State Traffic Plans Engineer					
rawn By		3-76	Revision	Sheet No.	Index No.			
hecked By		3-76	00	l of l	17350			

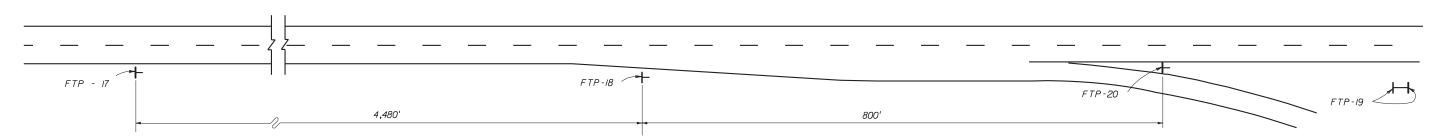
STATE OF FLORIDA WELCOME CENTER 1 MILE

STATE OF FLORIDA WELCOME CENTER

STATE OF FLORIDA
OFFICIAL
WELCOME CENTER



 Sign No. FTP-17
 Sign No. FTP-18
 Sign No. FTP-19
 Sign No. FTP-20



Note: Roadway not drawn to scale

Distances shown are adequate for driver communication
but may be altered slightly if conditions require.

Tourist Information Center NEXT RIGHT

Sign No. FTP-21

Note: Sign FTP-2I shall be used as a supplemental guide sign at interchanges which have a Tourist Information Center approved for such signing (locate half-way between normal guide signs)

Notes :

- (1) Signs and sign structures shall be erected in accordance with the details shown on Index 9535.
- (2) Sign FTP-19 shall be located on the Welcome Center grounds in proximity to the building and as far from the main line roadway as possible (2 signs back to back).
- (3) Sign FTP-I7, I8, I9 shall be located on limited access highways only.
- (4) All legend to be Series E.
- (5) See Index 17355 for sign details.

FOR LIMITED ACCESS HIGHWAYS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

WELCOME CENTER SIGNING

	Names	Dates	Approve	d By Clark	0 1 41
Designed By		6-75	Sta	Engineer	
Drawn By		6-75	Revision	Sheet No.	Index No.
Checked By		6-75	00	1 of 2	17351

STATE OF FLORIDA WELCOME CENTER

1 MILE

SIGN NO. FTP-22A

STATE OF FLORIDA STATE

SIGN NO. FTP-19

1/2 MILE

SIGN NO. FTP-22B

SIGN NO. FTP-23

FTP - 22A

FTP - 22B

FTP - 23

*
800'

*
800'

*
800' Maximum For Rural Conditions
50' Minimum For Cangested Areas

*
800 Maximum For Cangested Areas

Notes

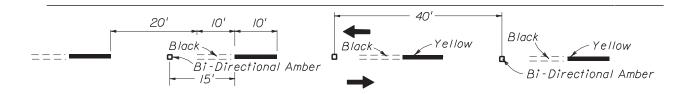
- (I) Signs and sign structures shall be erected in accordance with the details shown on Index 9535.
- (2) Sign FTP-19 shall be located on the Welcome Center grounds in proximity to the building and as far from the Main Line Roadway as possible (2 signs back to back)
- (3) All legend to be Series E.

FOR PRIMARY HIGHWAYS

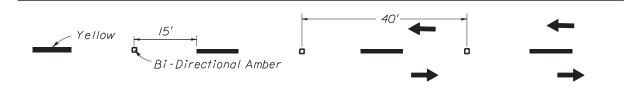
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

WELCOME CENTER SIGNING

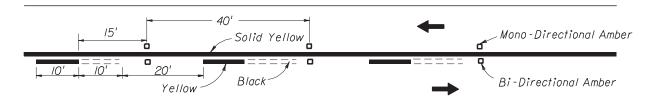
	Names	Dates	Approve		0 / 2/
Designed By		6-75	Clark a Acold State Traffic Plans Engineer		
Drawn By		6-75	Revision	Sheet No.	Index No.
Checked By		6-75	00	2 of 2	17351



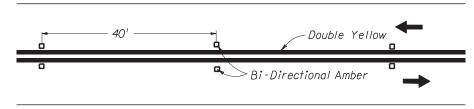
ALTERNATING SKIP LINE



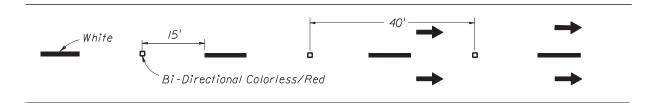
SKIP LINE



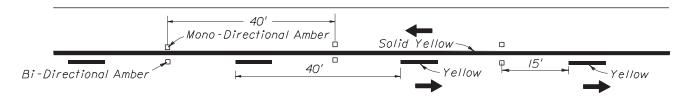
SOLID LINE WITH ALTERNATING SKIP



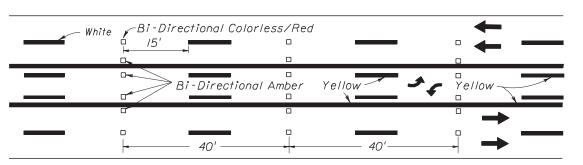
DOUBLE SOLID LINE



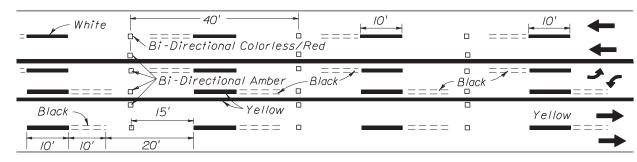
MULTI-LANE



SOLID LINE WITH SKIP



SKIP LINE WITH TWO WAY LEFT TURN LANE



ALTERNATING SKIP LINE WITH TWO WAY LEFT TURN LANE

- I. Reflective Pavement Markers shall be spaced at 40' on all skip lane lines and skip center lines. This spacing may be reduced to 20' if specifically called for in the plans.
- 2. The spacing on solid lines and solid/skip combination lines shall be 40'.
- 3. All R.P.M.s shall be offset I" from solid lines.
- 4. These spacings may be reduced for sharp curves if required.
- 5. All R.P.M.s shall be class "B".

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

TYPICAL PLACEMENT OF REFLECTIVE PAVEMENT MARKERS

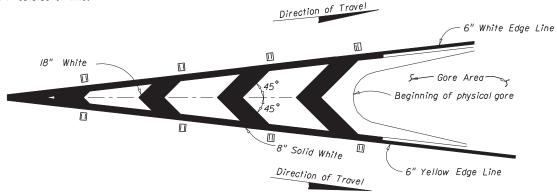
	Names	Dates	Approve		0 / 2/							
Designed By		10-87	Sta	Clark a Scott State Traffic Plans Engineer								
Drawn By			Revision	Sheet No.	Index No.							
Checked By		10-87	00	1 of 2	17352							

NOTE

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Raised pavement markers shall be set I" outside of line.

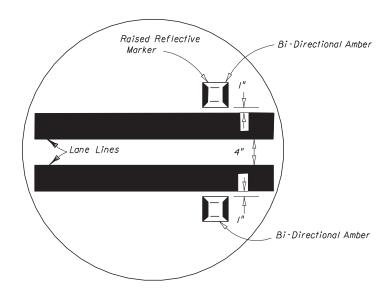


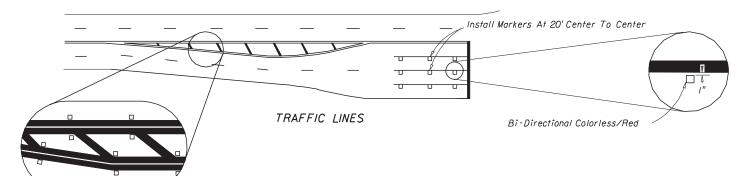
RPM PLACEMENT FOR TRAFFIC CHANNELIZATION AT GORE (TRAFFIC FLOWS IN SAME DIRECTION)

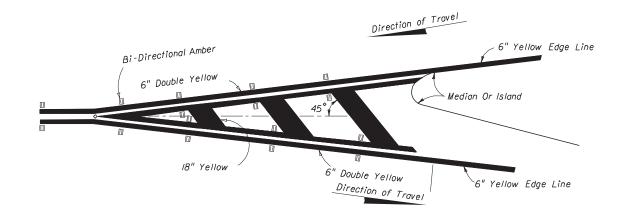
NOTE

Reflective Pavt. Markers To Be BI-Directional Amber

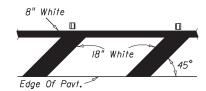
Raised pavement markers (Bi-Directional Colorless/Red) should be used in all gores of this type







RPM PLACEMENT FOR TRAFFIC SEPARATION (TRAFFIC FLOWS IN OPPOSITE DIRECTION)



PLACEMENT OF RPMS ON SHOULDER MARKINGS

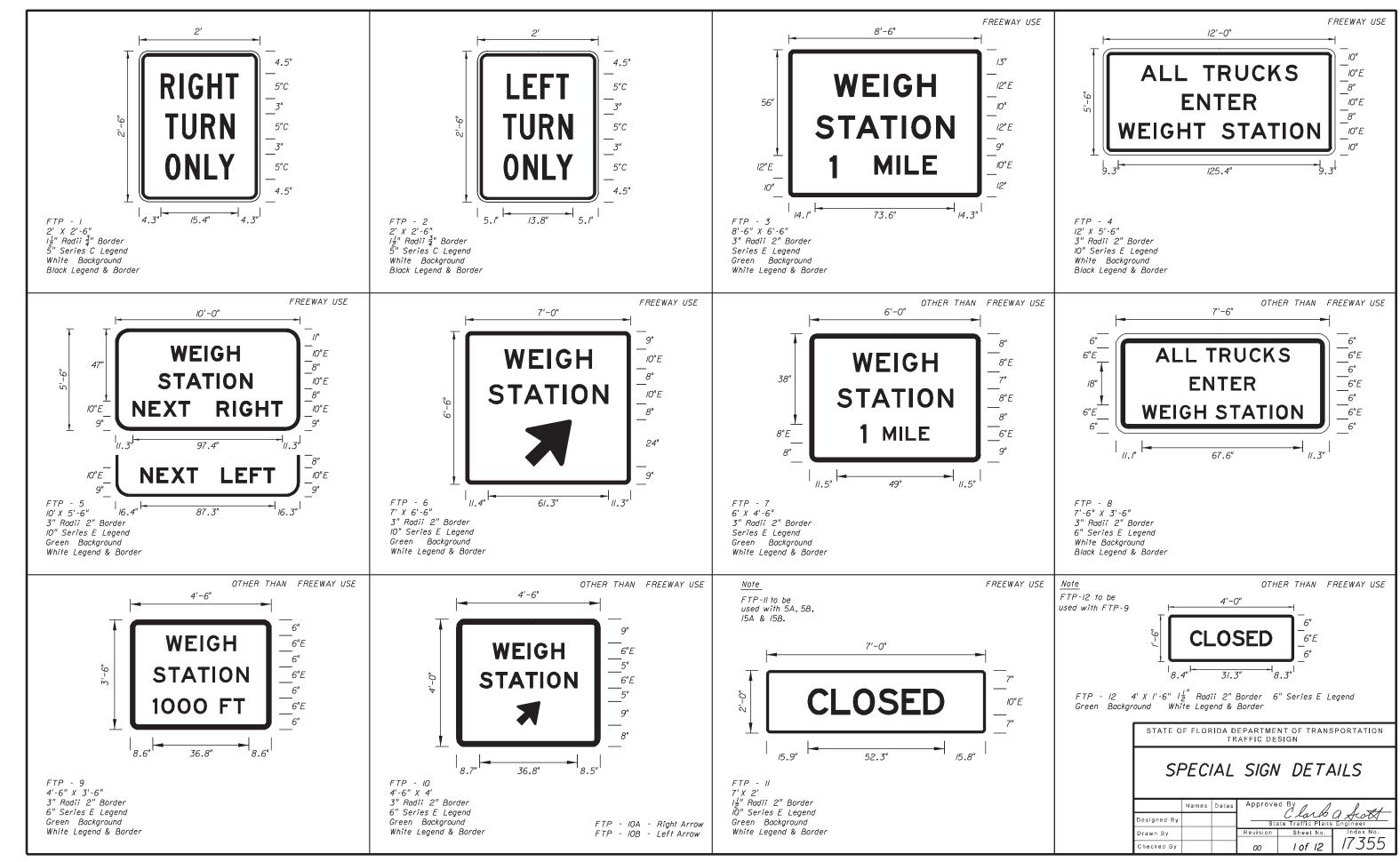
Shoulder Markings For Left Side Of Roadway Shall Be Yellow.

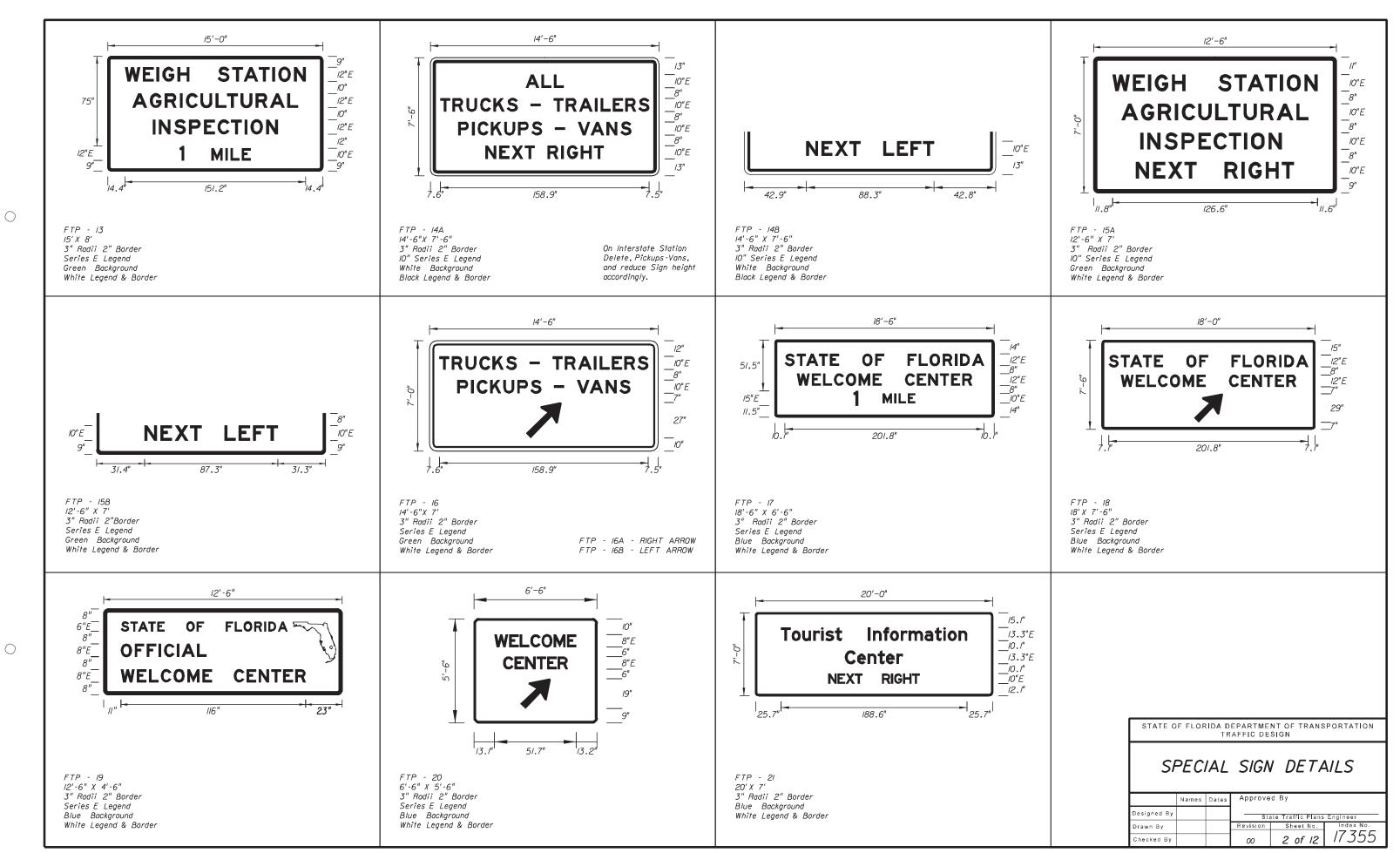
For Placement Of RPMS On Ramps See Index 17345.

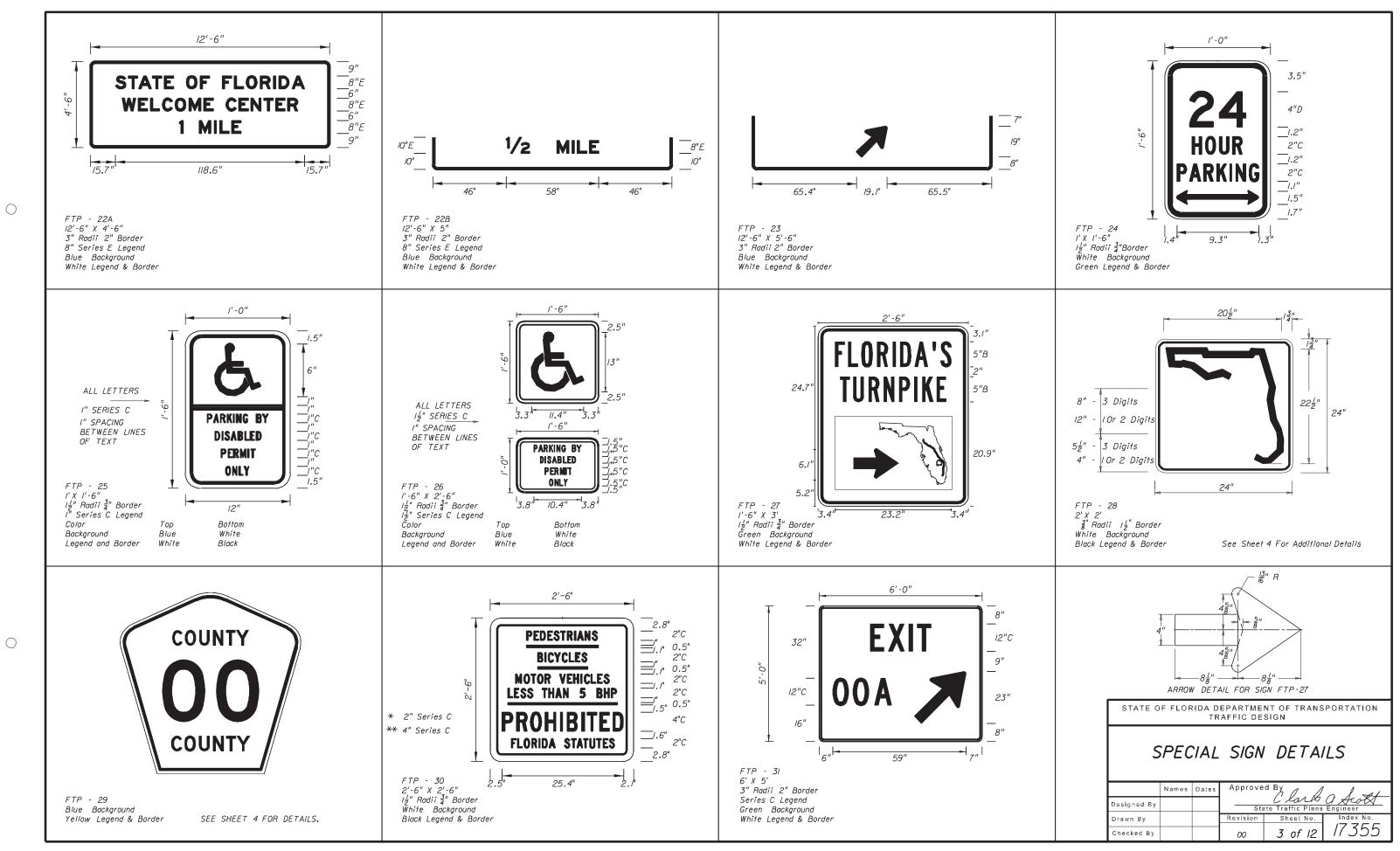
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

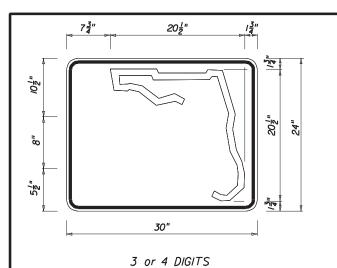
TYPICAL PLACEMENT OF REFLECTIVE PAVEMENT MARKERS

	Names	Dates	Approve		
Designed By		10-75	Sta	Clark ate Traffic Plans	C Acold Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By		10-75	00	2 of 2	17352

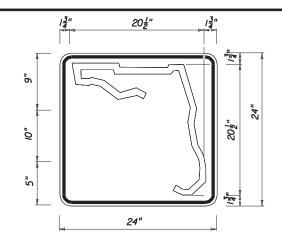








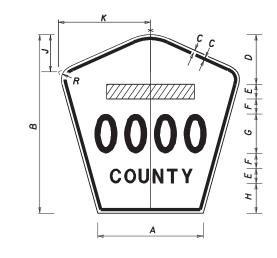
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1 or 2 DIGITS

NUMERAL SIZE

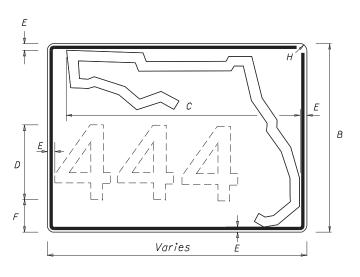
DIGITS	NUMERAL SIZE (mm)	SERIES	PANEL SIZE (mm)
1-2	10"	D	24" x 24"
3	8"	С	24" x 30"
4	8"	С	24" x 30"

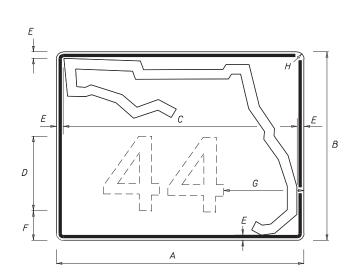


Notes :

- I. All Legend Series "D".
- 2. Color: Yellow Legend and Border on Blue Background.
- 3. When used on a guide sign, marker must be overlayed on a rectangular Yellow Background as shown in chart. **

INDEPENDENT USE OTHER THAN FREEWAY





3 OR MORE DIGITS

I OR 2 DIGITS

Α	В	С	D	Ε	F	G	Н
30"	24"	26"	12"	/ <u>/</u> "	2 3 "	8 <u>1</u> "	/ <u>/</u> "
36"	30"	32"	/5"	1 <u>1</u> "	3 <u>/</u> "	8 <u>3</u> "	/ <u>/</u> "
42"	36"	38"	<i>1</i> 5"	/ <u>/</u> "	6 <u>/</u> "	//"	/ <u>/</u> "

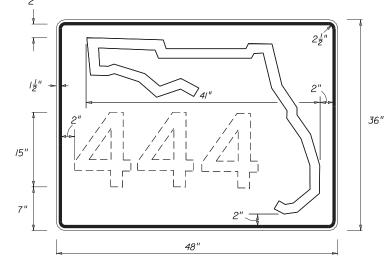
GUIDE SIGN USE

- Notes: I. Florida marker shall have Black Legend with White Background.
 - 2. Stroke width of State outline to be I" for independent use and l_4^{II} for Guide Sign.
 - 3. Numbers are series D.

FLORIDA ROUTE MARKER FTP - 28

SIGN						DIME	NSIONS	5					
31014	Α	В	С	D	Ε	F	G	Н	J	κ	R	S	**
1 & 2 DIGIT POST MOUNTED	14 <u>3</u> "	24"	<u>!</u> "	5"	2"	2"	8"	3"	5 <u></u> #"	12 <u>3</u> "	/ <u>/</u> "	5 <u>5</u> "	
3 DIGIT POST MOUNTED	17 <u>3</u> "	30"	<u>!</u> "	7 <u>4</u> "	2 <u>/</u> "	2 <u>1</u> "	8"	4 <u>3</u> "	6 <u>/</u> "	15 <u>1</u> "	/ <u>/</u> "	6 <u>5</u> "	
4 DIGIT POST MOUNTED	21 <u>4</u> "	36"	<u>!</u> "	8"	3"	3"	8"	8"	7 <u>/</u> 2"	18 <u>1</u> "	/ <u>3</u> "	7 <u>/</u> "	
2 DIGIT OVERHEAD	214"	36"	<u>3</u> "	8"	3"	3"	12"	4"	7 <u>/</u> "	18 <u>1</u> "	/ <u>3</u> "	7 <u>/</u> "	* * * 40"x 41"
3 DIGIT OVERHEAD	29 <u>‡</u> "	36"	<u>3</u> "	8"	3"	3"	12"	4"	8"	21 3 "	24"	8 <u>/</u> "	* * * 40"x 44"
4 DIGIT OVERHEAD	36 3 "	42"	<u>3</u> "	//"	3"	3"	12"	7"	10 <u>1</u> "	26"	2 4 "	8 <u>!</u> "	*** 42"x 52"

COUNTY ROUTE MARKER DETAIL FTP - 29



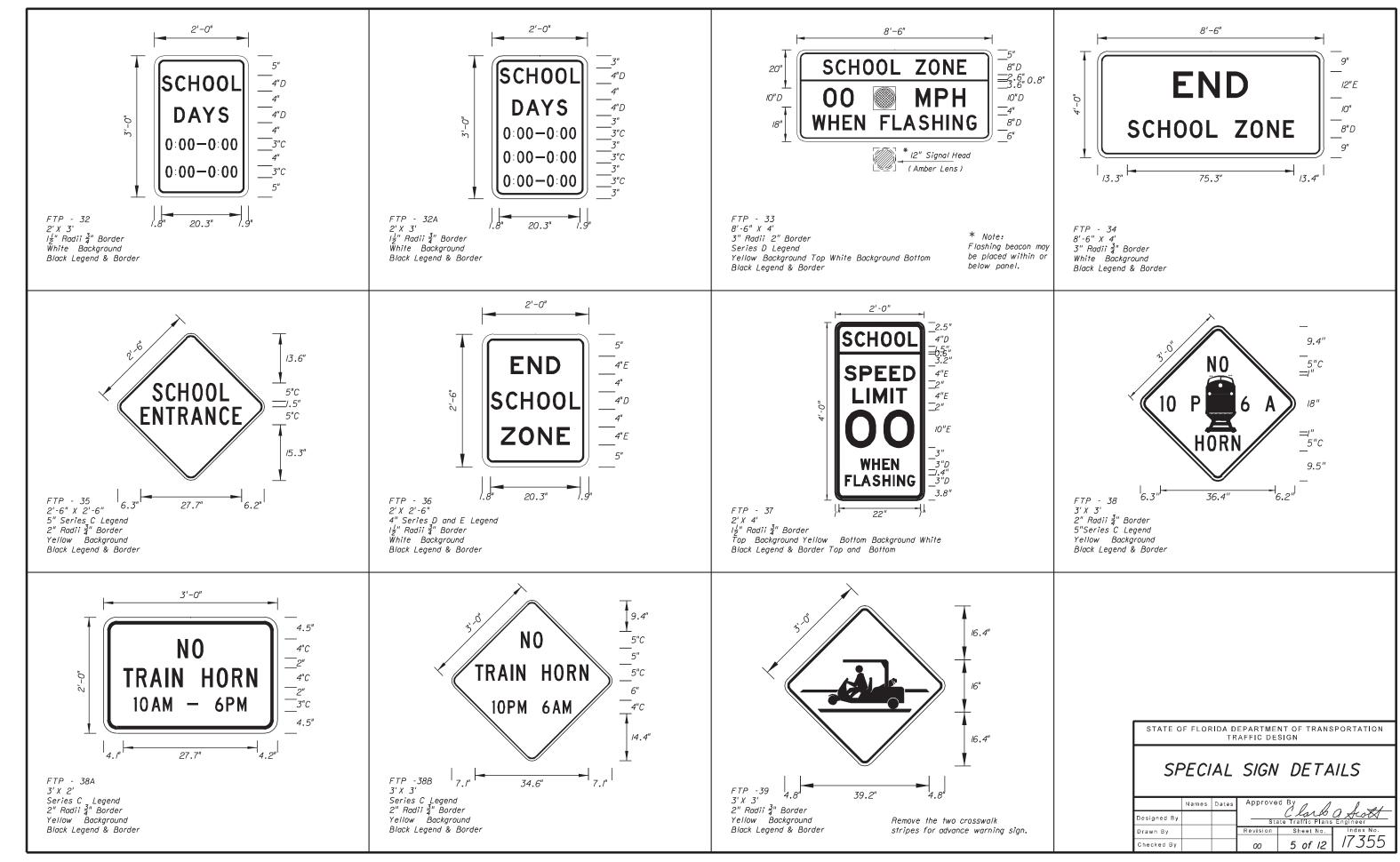
I-3 DIGITS 375 SERIES C 4 DIGITS 300 SERIES C

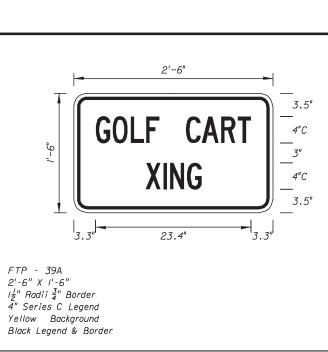
INDEPENDENT USE FOR FREEWAY

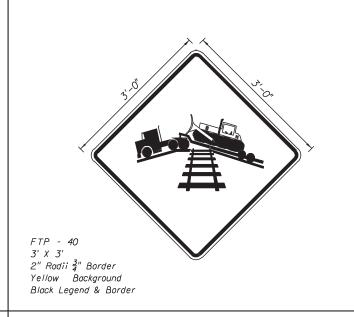
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

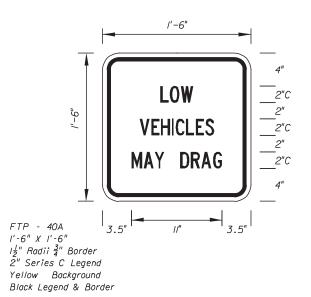
SPECIAL SIGN DETAILS

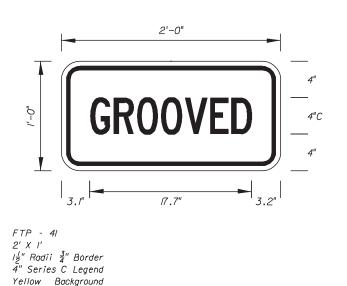
	Names	Dates	Approve		2 l 21
Designed By			Sta	Clark ate Traffic Plans	C ACOCT Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	4 of 12	17355

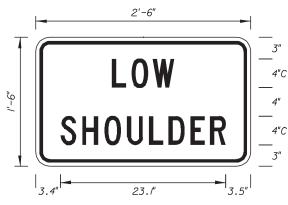






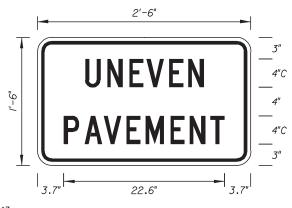








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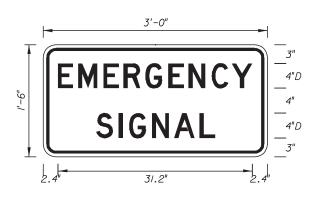


FTP - 43 2'-6'' X I'-6'' $I_Z^{f''}$ Radii $\frac{3}{4}$ " Border 4" Series C Legend Yellow Background Black Legend & Border



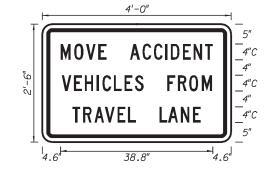
2'-6" X 3'
|\frac{1}{2}" Radii \frac{3}{4}" Border

Series E Legend
Yellow Background Top
Black Legend & Border

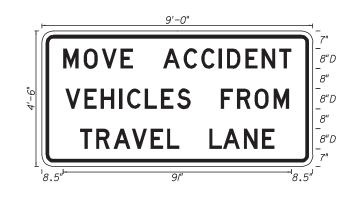


FTP - 45 3' X /'-6" $I_2^{\frac{1}{2}}$ " Radii $\frac{3}{4}$ " Border 4" Series D Legend Yellow Background Black Legend & Border

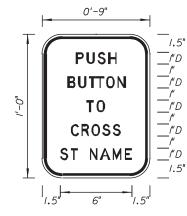
Black Legend & Border



FTP - 46 4' X 2'-6" 2" Radii ¾" Border 4" Series C Legend White Background Black Legend & Border



FTP - 46A 9'X 4'-6" 3" Radii ¾" Border 8" Series D Legend White Background Black Legend & Border



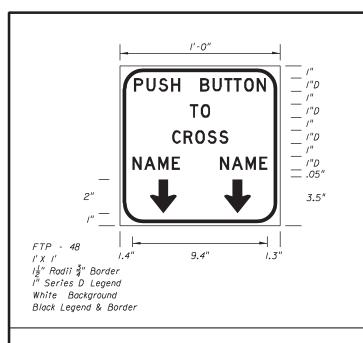
FTP - 47
9" X 12" $I_2^{L''}$ Radii $\frac{3}{4}$ " Border
1" Series D Legend
White Background
Black Legend & Border

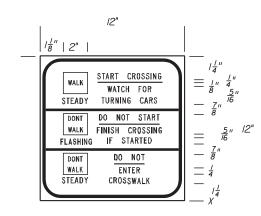
FTP - 44

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

SPECIAL SIGN DETAILS

	Names	Dates	Approve		a 1 se
esigned By			Sta	Clark ate Traffic Plans	C Acolor Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	6 of 12	17355

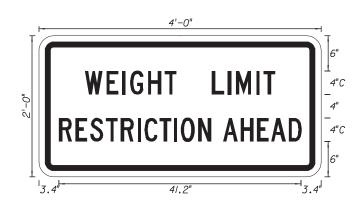




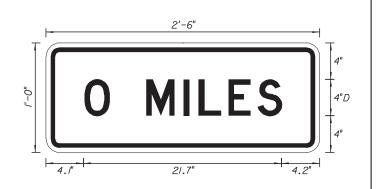
FTP - 49
I'X I'
I'2 Radii 3 Border
Series C Legend
White Background
Black Legend & Border

Notes for FTP 49:

- I. Text for FTP 49 shall be $\frac{1}{2}$ " except (WALK) and (DONT WALK) which will be $\frac{7}{16}$ ".
- 2. Spaceing between lines of legend shall be $\frac{5}{16}$ " except as noted.
- 3. Underbar spacing as detailed.
- 4. Colors shall be White background with Black legend and border Walk Plaque White legend on Black background Dont Walk Plaque Orange legend on Black background The international symbol may be used for walk and don't walk



FTP - 50 5'X 2' $l_Z^{\#}$ Radii $\frac{3}{4}$ " Border 4" Series C Yellow Background Black Legend & Border

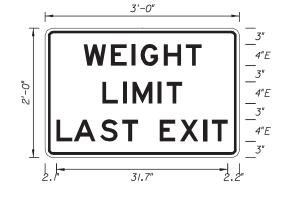


FTP - 5I 3'XI' $I_{Z}^{I'}$ Radii $\frac{3}{4}$ " Border 4" Series D White Bockground Black Legend & Border

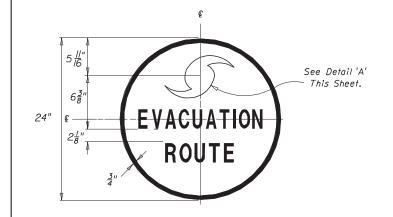
Black Legend & Border

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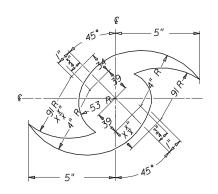
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FTP - 52 3'X 2' $I_2^{L''}$ Radii $\frac{3}{4}''$ Border 4'' Series E White Background Black Legend & Border

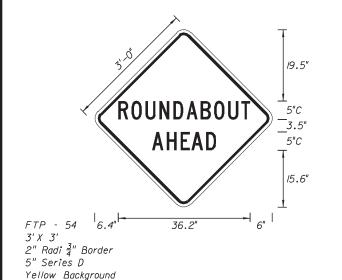


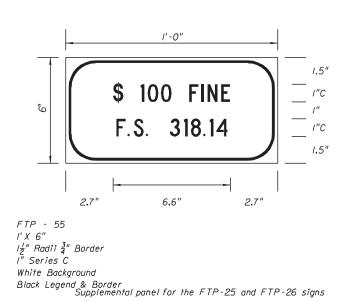
FTP - 53
2' Diameter
3" Border
3" Series C
Blue Background
White Legend , Border & Symbol

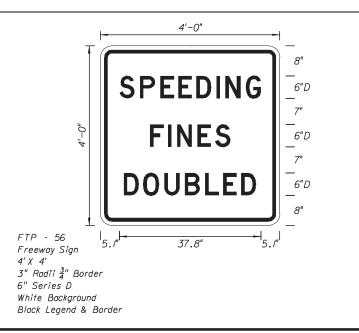


DETAIL 'A ' for FTP - 53

Symbol



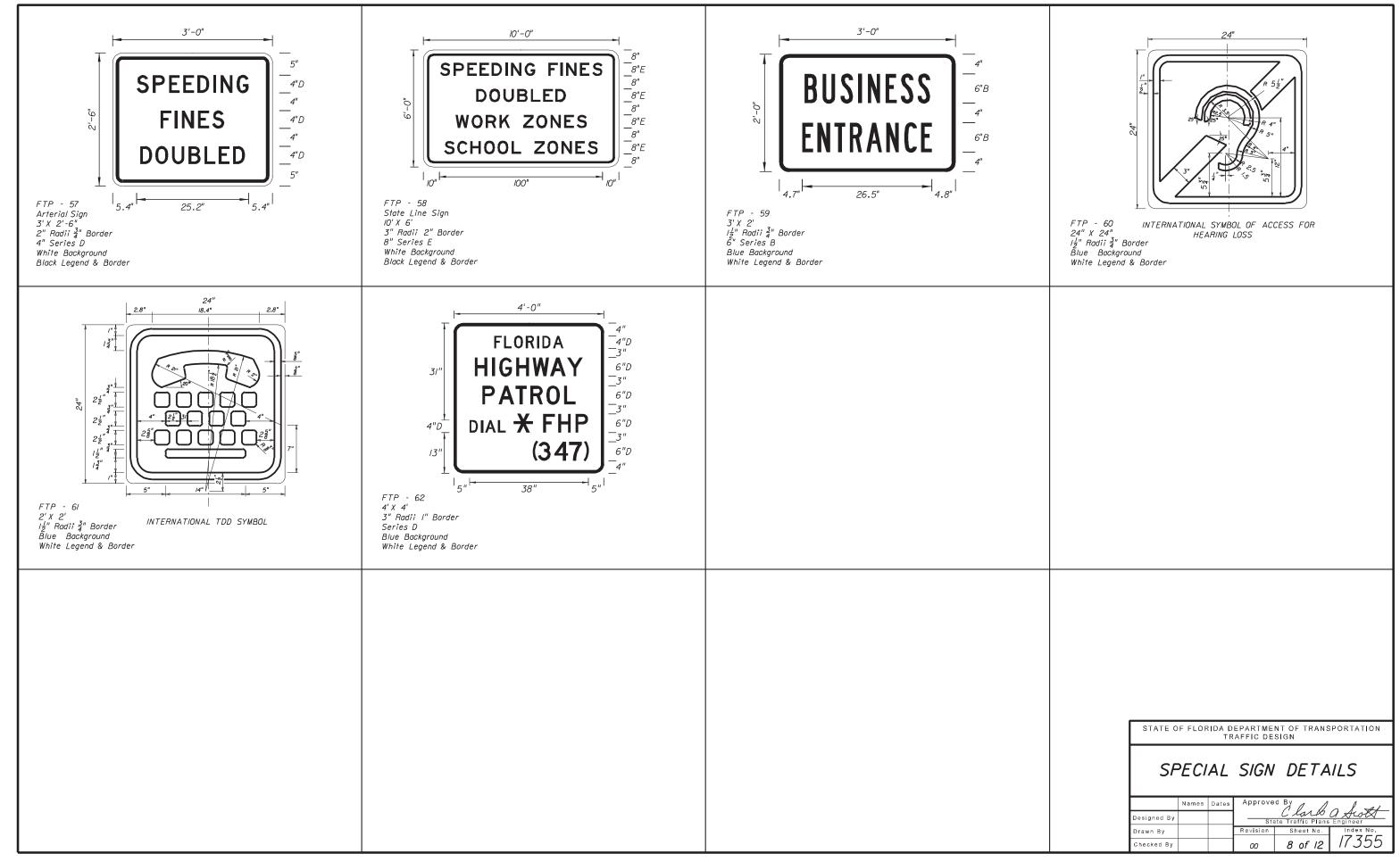


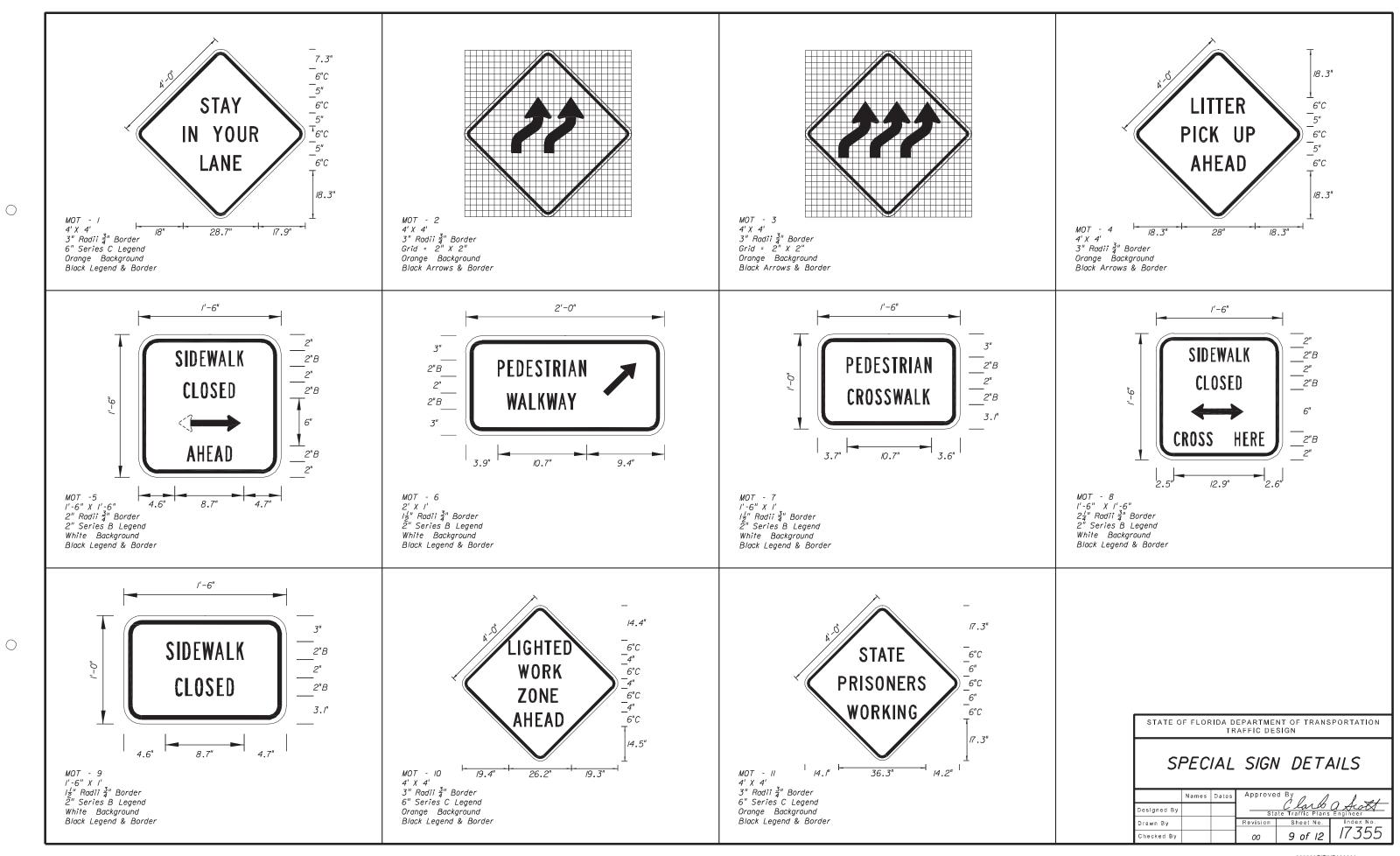


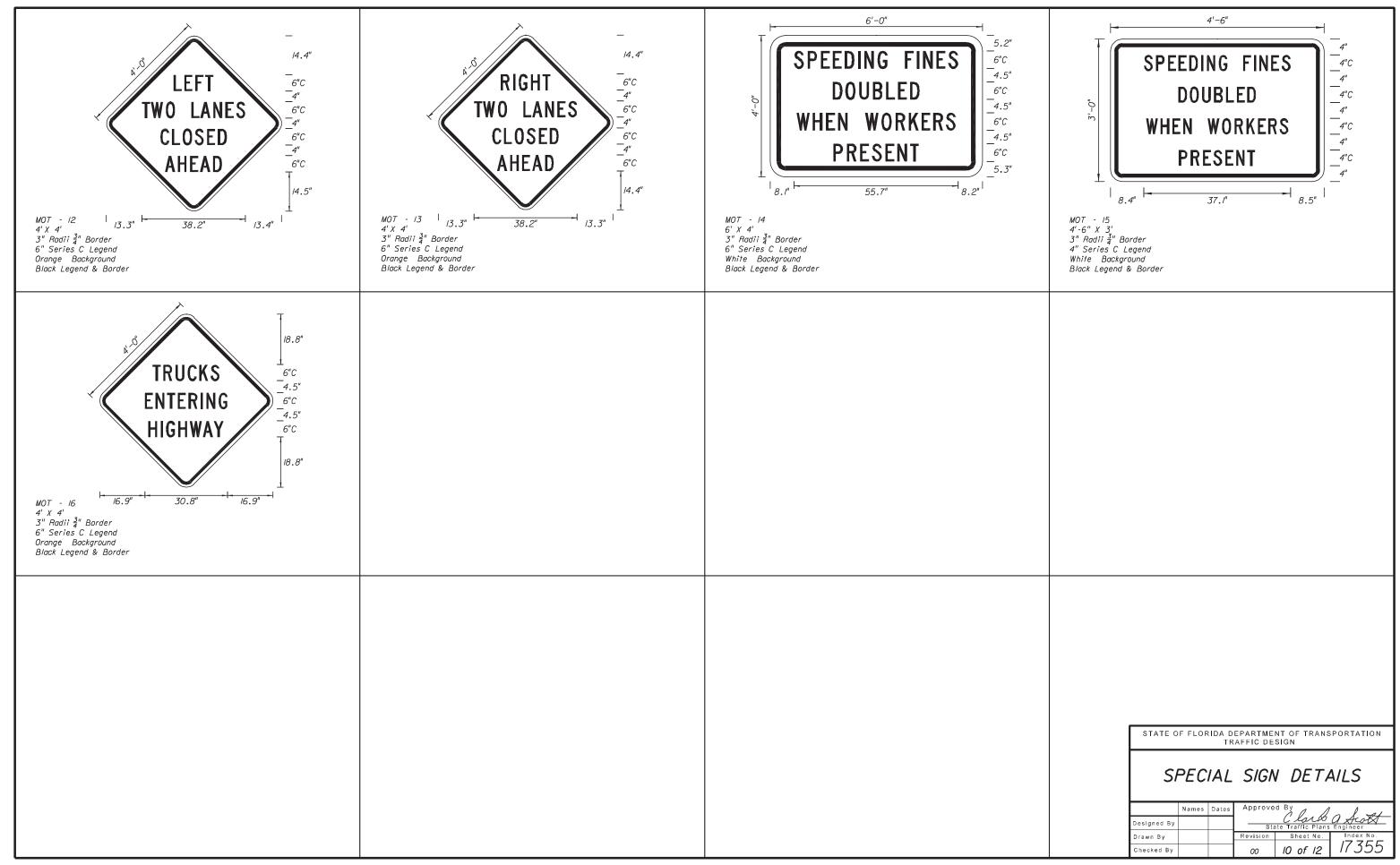
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

SPECIAL SIGN DETAILS

	Names	Dates	Approve	à // //	
Designed By			Sta	ate Traffic Plans	C Acold Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	7 of 12	1/355





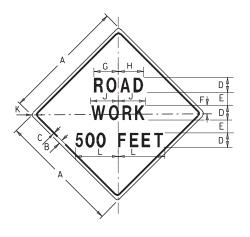




G20-1 5' X 2' 1.5 Radii .75 Border 6" Series C Legend Orange Background Black Legend & Border

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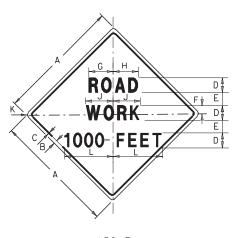
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W20-IA

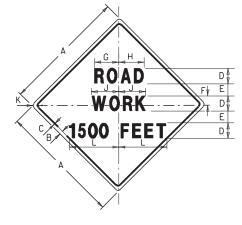
LEGEND AND BORDER: BLACK
BACKGROUND: ORANGE

	DIMENSIONS IN INCHES													
A B C D E F G H J K L														
36	<u>5</u> 8	<u>7</u> 8	5D	3 <u>/</u>	3 <u>/</u>	8 <u>3</u>	8 <u>7</u>	9	2 <u>/</u>	17 1/4				
48	<u>3</u>	14	7D	4 <u>3</u>	4 <u>1</u>	// <u>//</u> 6	12 <u>7</u>	12 5	3	24				



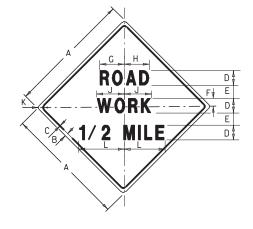
W20-IB
LEGEND AND BORDER: BLACK
BACKGROUND: ORANGE

	DIMENSIONS IN INCHES													
A B C D E F G H J K L														
36	5 8	<u>7</u>	5D	3 <u>/</u> 2	34	8 <u>3</u>	8 7	9	24/	18 3				
48	<u>3</u>	/ /	7D	4 3	4 <u>/</u> 2	// <u>//</u>	12 5	12 <u>5</u>	3	25 7				



W2O-IC
LEGEND AND BORDER: BLACK
BACKGROUND: ORANGE

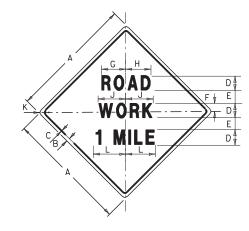
	DIMENSIONS IN INCHES											
Α	В	С	D	E	F	G	Н	J	К	L		
36	<u>5</u> 8	<u>7</u>	5D	3 <u>/</u>	34	8 <u>3</u>	8 7 /8	9	24	18 3		
48	<u>3</u>	14	7 <i>D</i>	4 <u>3</u>	4 <u>/</u> 2	// <u>//6</u>	12 <u>7</u>	12 <u>5</u>	3	25 ¾		



W20-ID

LEGEND AND BORDER: BLACK
BACKGROUND: ORANGE

	DIMENSIONS IN INCHES											
Α	В	С	D	E	F	G	Н	J	К	L		
36	<u>5</u>	<u>7</u>	5D	3 <u>/</u>	34	8 <u>3</u>	8 7 /8	9	21/4	16 <u>3</u>		
48	<u>3</u>	14	7D	4 <u>3</u>	4 <u>/</u> 2	// <u>//</u> 6	12 <u>7</u>	125	3	22		



W20-IE

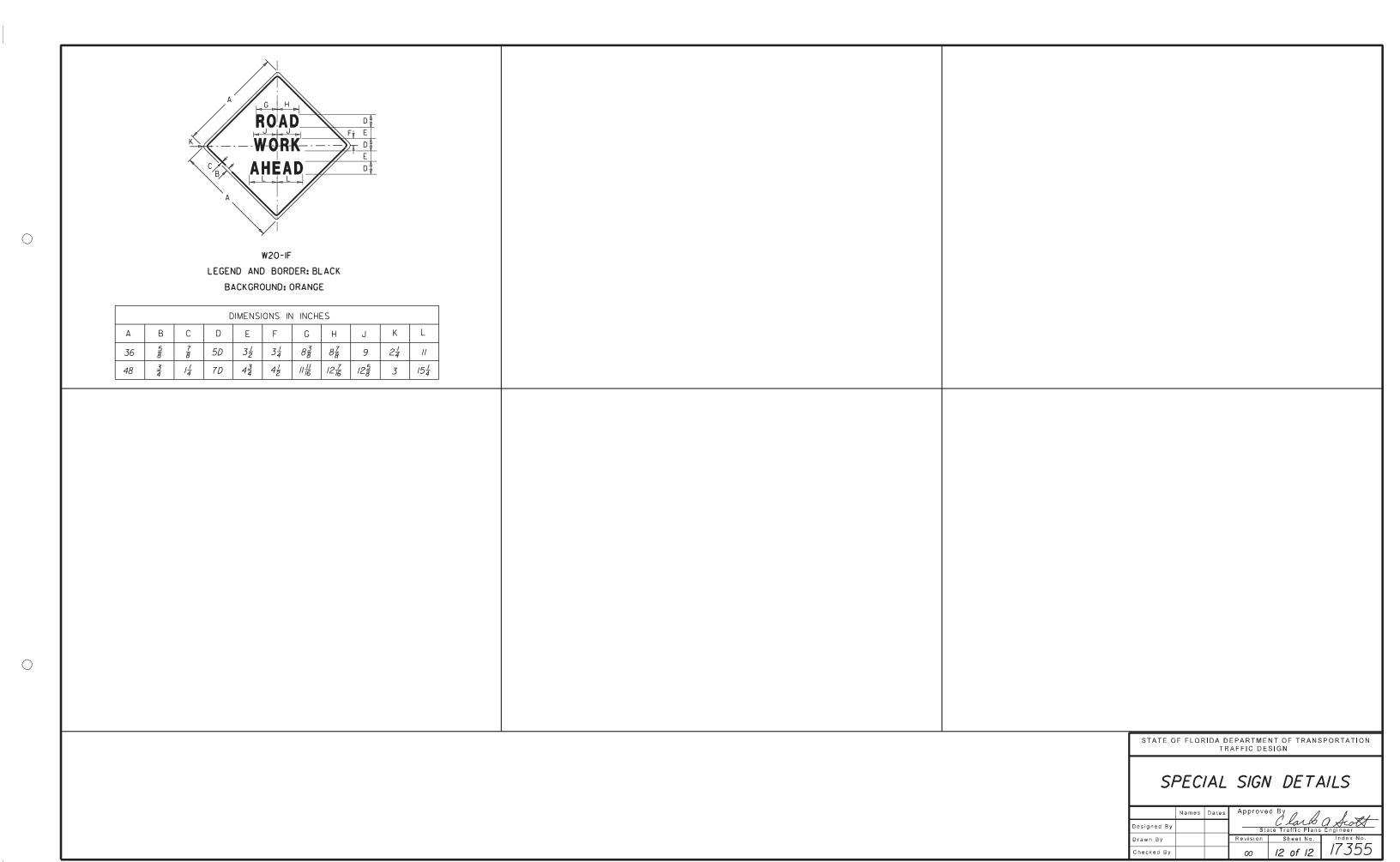
LEGEND AND BORDER: BLACK
BACKGROUND: ORANGE

	DIMENSIONS IN INCHES										
А	В	С	D	E	F	G	Н	J	К	L	
36	<u>5</u>	<u>7</u>	5D	3 <u>/</u>	34	8 <u>3</u>	8 <u>7</u>	9	2 1/4	// 3 /8	
48	<u>3</u>	1/4	7D	43/4	4 <u>/</u> 2	// <u>//</u> 6	12 <u>5</u>	12 <u>5</u>	3	15 <u>7</u>	

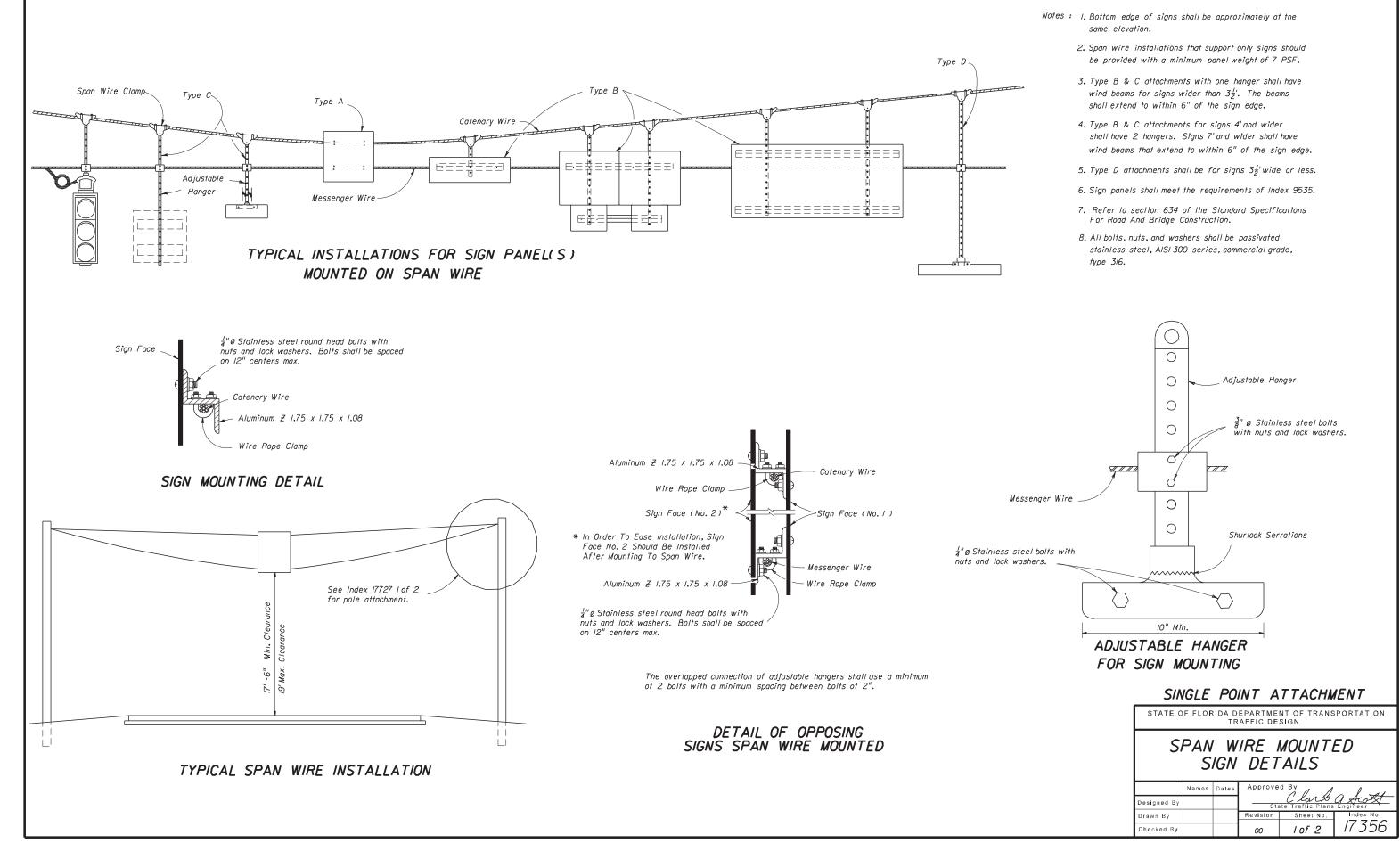
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

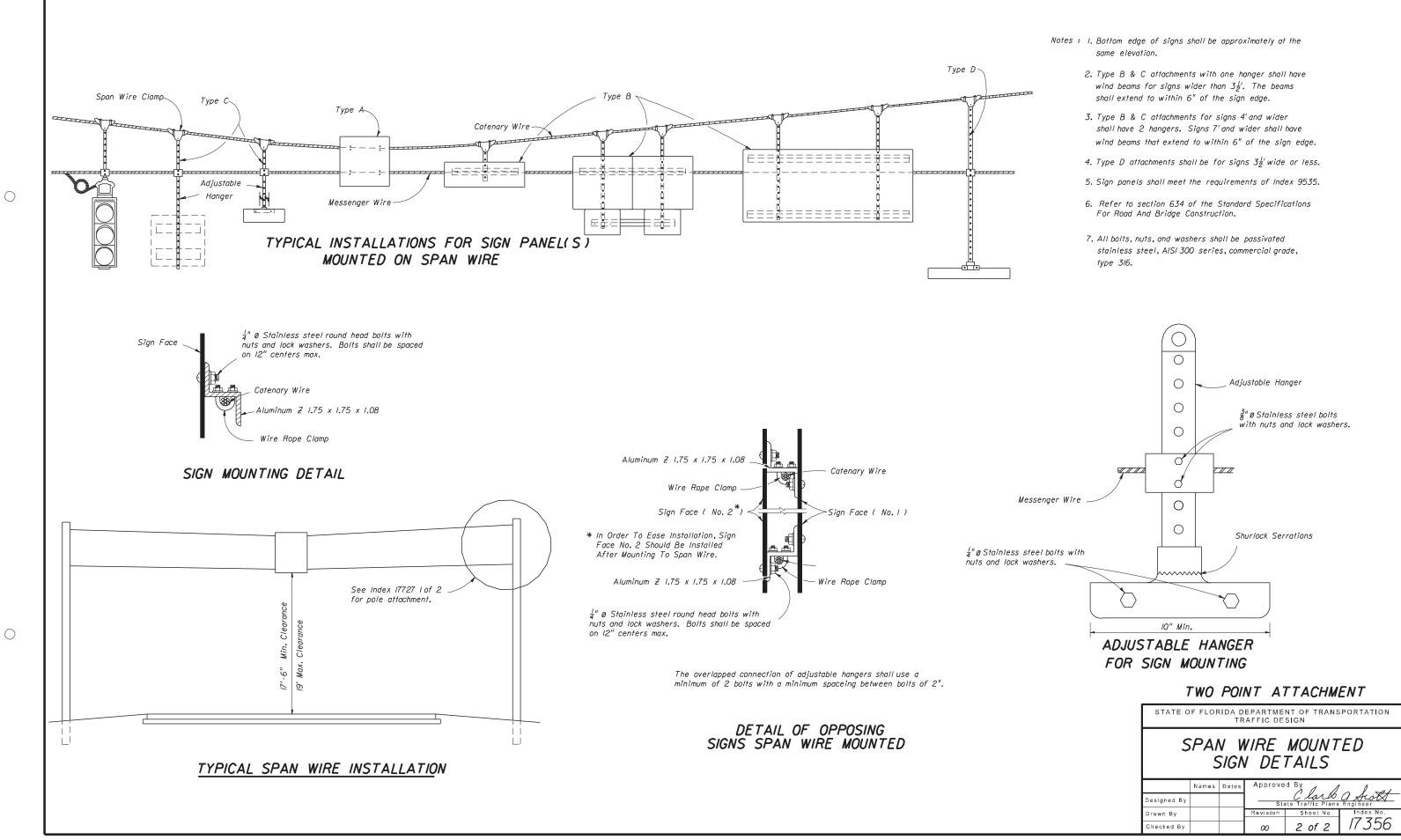
SPECIAL SIGN DETAILS

	Names	Dates	Approve	<i>i</i> // //	0 1 21
Designed By			Sta	C Lando, ate Traffic Plans	Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	II of 12	<i>17355</i>



\$\$\$\$\$\$\$YTIME\$\$\$\$\$







SIGN LOCATIONS TYPICAL

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WEIGHT LIMIT RESTRICTION AHEAD

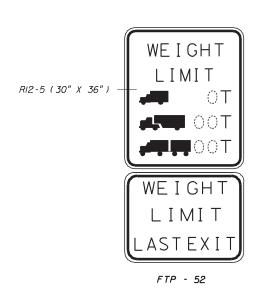
FTP - 50

NOTE

- I. See Standard Highway Signs dated 1979 for sign RI2-5 detail.
- 2. Sign location No. 3 may require some field adjustment.
- 3. Signs FTP-50, FTP-51 & FTP-52 shall have a $\frac{3}{2}$ " edge and $\frac{3}{4}$ " border with a 4" radius.
- 4. The Cross Road is the last detour around the restricted bridge.
- 5. Sign location No. 2 should be established from the Cross Road the following approximate distances; Interstate I Mile Non-Interstate I/2 Mile.
- 6. See Index 17355 for sign details.

WE I GHT
LIMIT
OT
OT
OT
OT
OT
FTP -51

2



3

WEIGHT
LIMIT
OT
OT
OT
OT

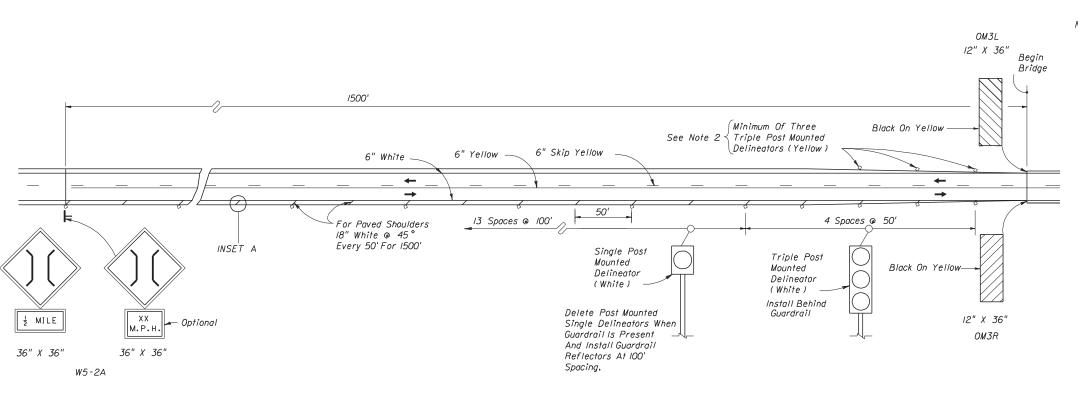
4

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

BRIDGE WEIGHT RESTRICTIONS

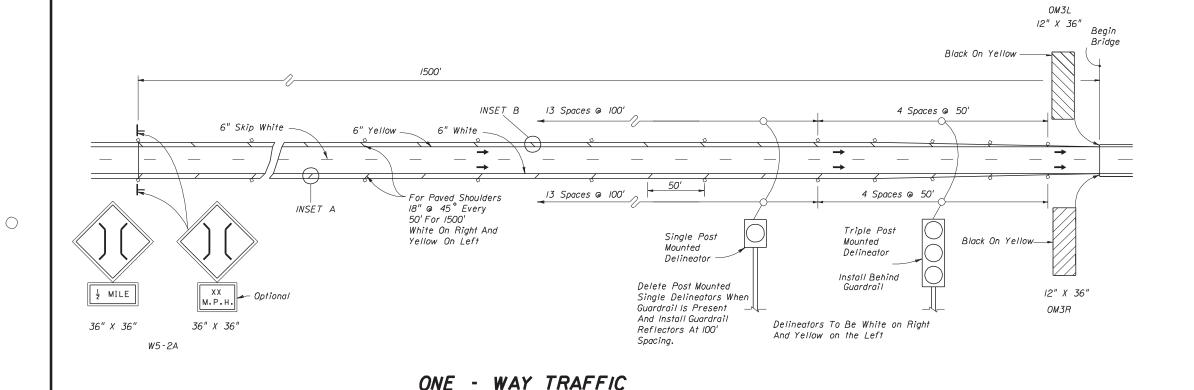
	Names	Dates	Approve		
Designed By			Sta	Clark ate Traffic Plans	
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	I of I	1/35/

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TWO - WAY TRAFFIC

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NOTES:

- I. Bridges should be marked as narrow bridges under the following conditions:
 (I) For approach roadways with poved shoulders when the bridge width including shoulders is less than the width of the approach roadway including paved shoulders.
- (2) For approach roadways without paved shoulders when the bridge shoulder width is less than 2'.
- Roadways with two-way traffic:
 (I) No passing zone should be extended 1500' in advance of narrow bridge.

(2) The post mounted delineators shall be installed on both sides of the roadway (White On Right / Yellow On Left) for a distance of 1500' in advance of a narrow bridge if the bridge or the approach is on a curve.

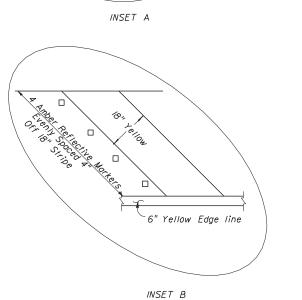
4. Delineators to be placed not less than 2' or more than 8' outside the outer edge of pavement.

3. Delineators on both sides of roadway

shall face traffic approaching bridge.

5. The OM-3R & OM-3L mounting height shall be 4'above the roadway edge.
The panels may be post mounted at the bridges.

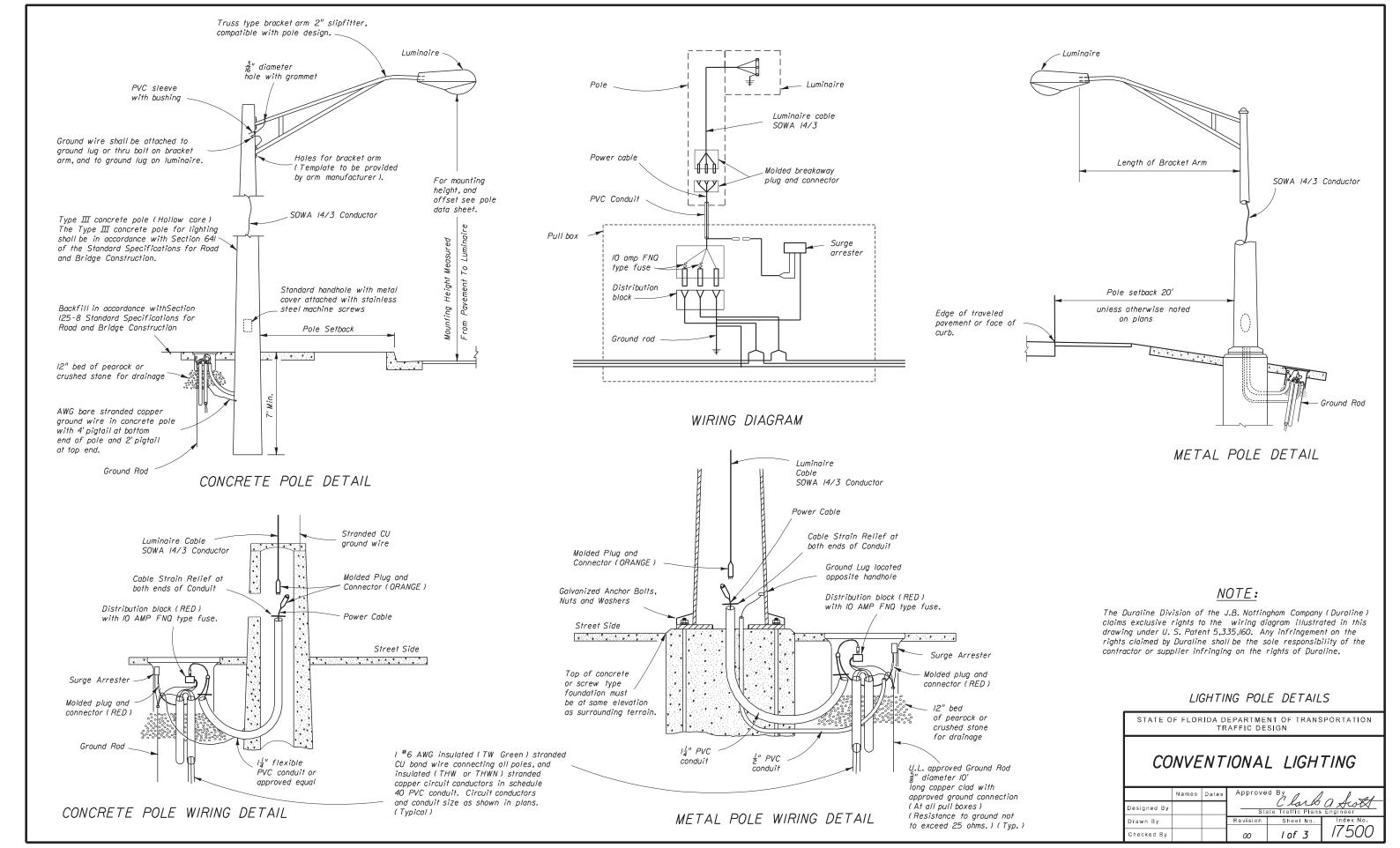
6" White Edge line

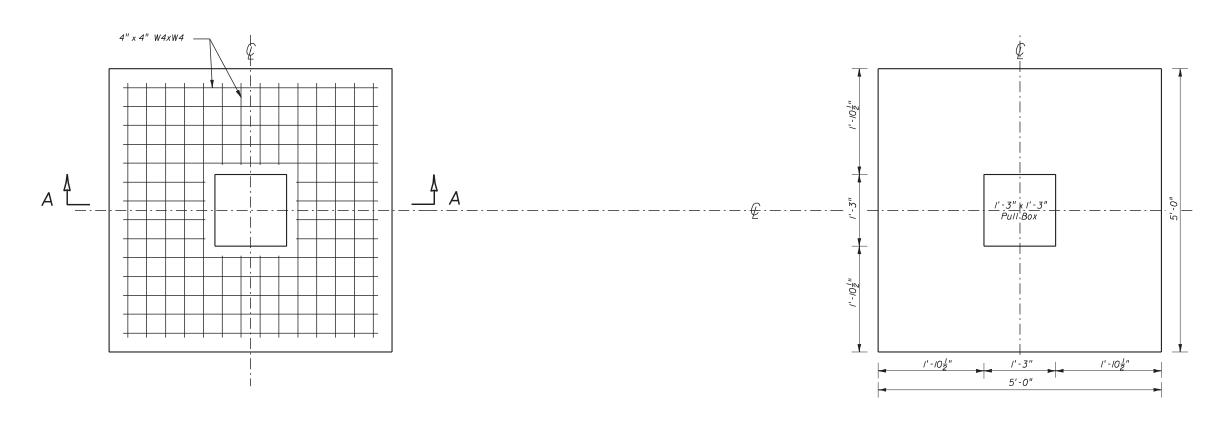


STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

RURAL NARROW BRIDGE TREATMENT

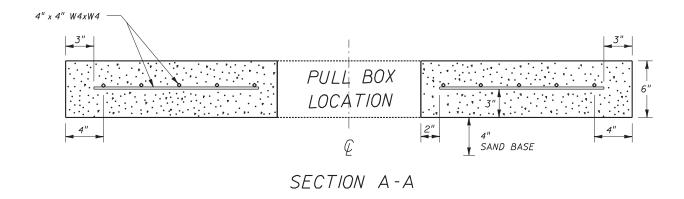
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REINFORCEMENT LAYOUT

SLAB DIMENSIONS



NOTES:

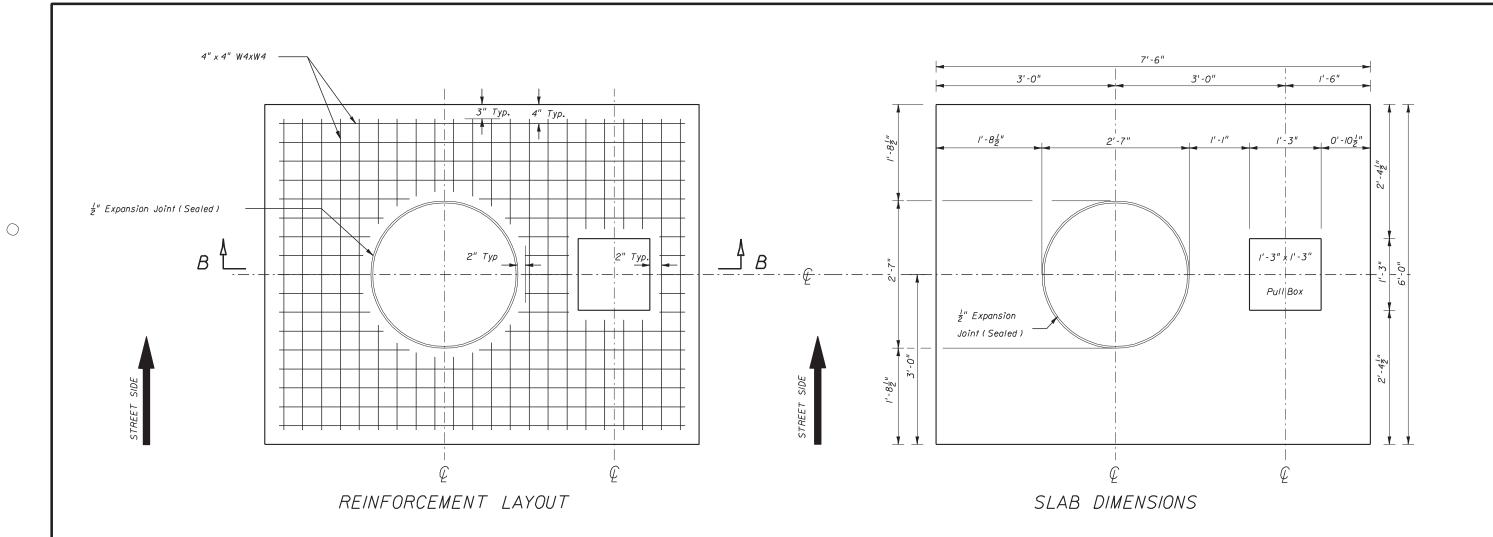
- I. Use clean free draining sand < 5% passing No. 200 sieve for base.
- 2. Welded wire fabric shall meet the requirements of ASTM A185.
- 3. Concrete strength at 28 days shall be f'c = 3 ksi
- 4. Outside edges of slab shall be cast against formwork.
- 5. The pull box shown is I'-3" x I'-3"; others approved under Section 635 of the Standard Specifications may be used.

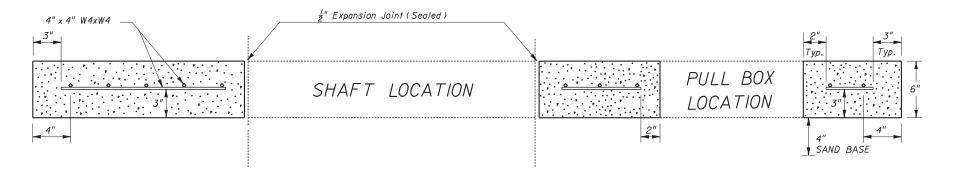
LIGHTING GENERAL NOTES AND SLAB DETAILS FOR PULLBOX LOCATIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

CONVENTIONAL LIGHTING

	Names	Dates	Approve			
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NOTES:

- I. Use clean free draining sand < 5% passing No. 200 sieve for base (4").
- 2. Welded wire fabric shall meet the requirements of ASTM A185.
- 3. Concrete strength at 28 days shall be f'c=3 ksi.
- 4. Outside edges of slab shall be cast against formwork.

SECTION B-B

- 5. The $\frac{1}{2}$ " thick expansion joint between shaft and slab shall be sealed with a hot poured elastic joint sealer.
- 6. Slabs to be placed around all Poles and Pull Boxes in rural locations. In urban areas or where space is limited slab dimensions may be adjusted as shown in the plans.
- 7. The pull box shown is I'-3" x I'-3"; others approved under Section 635 of the Standard Specifications may be used.

SLAB DETAILS FOR POLE AND PULL BOX LOCATIONS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

CONVENTIONAL LIGHTING

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- I) Ground rods shall have a resistance to ground not to exceed 25 ohms. Where the resistance is greater than 25 ohms, two or more ground rods connected in parallel shall be used. Contractor shall have necessary test equipment (current calibration certificate required) at final inspection to ensure acceptability of grounding system. Total grounding system not to exceed 10 ohms. All grounding system connections shall be exothermically welded. This includes all cable connections, ground rod connections, rod to rod connections, and splices. Method of Measurement and Basis of Payment as per Section 620 of the Standard Specifications.
- 2) The contractor shall be responsible for contacting all utility companies prior to any underground work. The utility company will locate and identify their facilities.

- 3) Contractor shall determine the service required date for the power company transformer installation at the pre-construction conference.
- 4) The power company reserves the right to install the riser, switch gear and weatherhead on power company poles at the expense of the contractor. Contact the power company for cost or for authorization for an alternate procedure.
- 5) Any damaged portions of galvanized steel poles and bracket arms shall be painted in accordance with Section 562 of the Standard Specifications.
- 6) Poles and bracket arms shall be designed in accordance with the design criteria, as indicated in the plans and using the applicable equations found in the AASHTO 'Standard Specifications For Structural Supports For Highway Signs, Luminaires And Traffic Signals'. The calculations shall be based on the actual projected area of the luminaire or 3.0 square feet whichever is greater.
- 7) The luminaire manufacturer shall place a permanent tag on the luminaire housing on which is imprinted the following information: Wattage, ballast type, lamp shown on design plans, lamp setting (position of luminaire), IES light distributation with this lamp in the position specified, input voltage and power factor. Luminaire photometric submittals required.
- 8) Before final acceptance, contractor shall provide 2 sets of full size as built plans to the maintaining agency.
- 9) Conduit routing shall be pole to pole, maintaining pole setback distance from edge of pavement. Any cable routing in locations where guardrail is proposed shall be 2'.
- 10) Pole positions and conduit routing may be adjusted, as approved by the Engineer, to prevent conflicts with utility and drainage structures not indicated, and prevent guardrail post conflict with underground lighting circuits.
- II) Where guardrail is constructed, the poles shall be placed a minimum of 4' behind the face of the guardrail.
- 12) Pole foundation installations shall be backfilled to the top of the foundation, compacted to a firm, stable condition approximately equal to that of the adjacent soil. The fill shall conform to existing grade and be fully sodded.
- 13) All splices shall be made in pullboxes or the pole base. No splices shall be made inside the conduit. The wires at pullboxes shall have sufficient length to completely remove connectors to the outside of pullboxes to make connectors accessible for changing fuses and trouble shooting the system.

- (4) Neutral wires to have white insulation. Do not use white or green insulated wires for ungrounded conductors.
- 15) Unless otherwise specified, all cable shall be single conductor, 98 percent conductivity stranded copper, with THW or THWN insulation
- 16) All exposed or surfaced mounted conduit shall be rigid or intermediate metal. These exposed runs of conduit shall be provided with either expansion joints or flexible metal conduit sections adequate to take care of vibrations and thermal expansions. All metal conduit shall be grounded. Steel conduit shall be hot dipped galvanized.
- 17) All conduit that will remain empty as spares shall be mandrel tested, cleaned inside and both ends capped. Leave the corrosion resistant pull/drag wire and place duct makers, or pullboxes to mark the location of the ends of the conduits.
- 18) Pull boxes shall be located at ends of conduit crossing roadways, and as necessary for the completion of the project.
- 19) These plans represent minimum acceptable criteria. The inspection per these drawings represent the minimum base of acceptance.
- 20) All material, unless otherwise specified, shall be Underwriters Laboratory approved.
- 21) Pull boxes shall meet the requirements of Section 635 of the 'Standard Specifications For Road And Bridge Construction' and Section 635 of the 'Minimum Specifications For Traffic Control Signals And Devices'.
- 22) A pull box shall be installed at each pole location. Pull boxes should be located 2' max from pole unless otherwise directed by the project engineer. Metal pull box covers shall be grounded. See General Requirements Section 635-4 of the Standard Specifications for Road and Bridge Construction.
- 23) At all pull boxes and pole bases, ends of conduit shall be sealed in accordance with Section 630 of the Standard Specifications for Road and Bridge Construction.
- 24) Luminaire shall be supplied with a regulator type ballast mounted on a hinged door or panel. The unit shall swing open to provide access to the ballast assembly by release of captive screws. The electrical connector shall be a quick disconnect plug. The unit shall be easily removed from the luminaire after release of the captive screws and disconnect plug.
- 25) All mounting heights are $\pm 2'$ -6" unless otherwise noted in plans.
- 26) A handhole is required in all poles. Handhole should be located opposite approaching traffic with cover fastened with Stainless Steel Screws. The handhole opening shall be at least 20 square inches.
- 27) The luminaire and arm on JOINT USE POLES shall be grounded.
- 28) Concrete slabs around poles and pull boxes shall be paid for under the contract unit price for Class I Concrete (Miscellaneous); the cost of reinforcing steel fabric shall be included in the price for Class I Concrete (Miscellaneous).

BREAKAWAY FEATURE

All conventional mounting height poles shall be mounted on a frangible metal base or system of breakaway couplings. If couplings are used, one coupling shall be provided for each anchor bolt connection. The only continuous connection of the pole to the foundation at each anchor bolt shall be provided by the couplings. The area between the top of the pole foundation and the base of the pole including the couplings shall be enclosed with a non-structural aluminum skirt.

If a frangible metal base is used, it shall be one piece and be designed to breakaway without the aid of any slipping or sliding surfaces.

The design of the breakaway feature shall be in accordance with the breakaway performance requirements of the AASHTO 'Standard Specifications For Structural Supports For Highway Signs, Luminaires and Traffic Signals'. The contractor (supplier) shall submit copies of test reports as evidence the breakaway feature meets the above specifications and calculations to verify the design will meet the AASHTO wind loading specified in the contract plans. No poles are to be installed prior to approval of submittal data.

Any substantial remains of a breakaway support, when it is broken away, should not project more than 4" as discussed in Section 7 of the above AASHTO specifications, and, Chapter 4, Section 4.2 of the AASHTO 'Roadside Design Guide'.

Poles behind bridge rail or barrier wall mounted, shall be non-frangible.

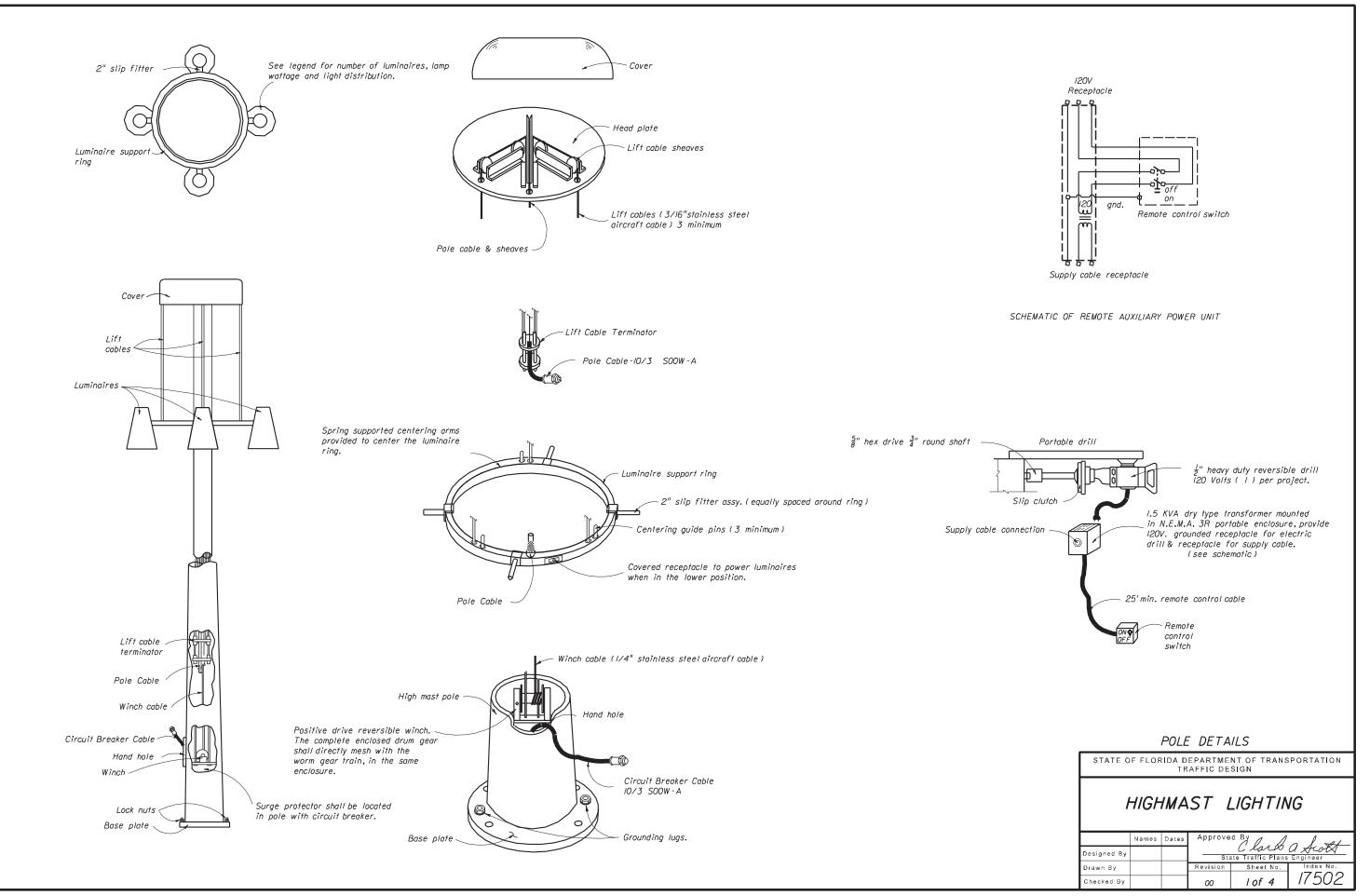
SURGE PROTECTOR SPECIFICATIONS

- 1. The unit shall withstand a surge current up to 20,000 Amps, and repetitive surges of 200 Amps for a minimum of 10,000 occurences.
- 2. The unit shall respond in less than 50 nanoseconds and within this time have a peak clamping voltage better than 1,100 Vrms.
- 3. The maximum allowable voltage that can pass continuously through the hot leg of the protector must be less than 550 Vrms.
- 4. The current drain shall be less than 100 microamps.
- 5. The unit shall be insulated 600 V. to ground and shall be weatherproof.
- 6. The unit shall not allow holdover current or conduction to ground after the surae ends.
- 7. Protection shall be achieved for both the 480 V. and neutral conductors with the surges being passed to ground and NOT to neutral.
- 8. There shall be no discharge lag in the protection of the 480 V. conductor over the neutral conductor.
- 9. Underwriters Laboratory approval not required.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

HIGHWAY LIGHTING GENERAL NOTES

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LUMINAIRE SPECIFICATIONS

The reflector with its aluminum cover shall be firmly attached to a cast ring. This ring shall have keyhole slots in its upper surface such that the reflector/refractor assembly may be readily attached to, or detached from, the luminaire bracket entry and lamp support assembly without completely removing the support bolts.

Each luminaire shall contain an integral auto-regulator type ballast connected for 480 volts input ± 10% and a power factor of more than 90%. The luminaire ballast shall be enclosed within an aluminum housing which intregally attaches to the luminaire bracket entry and lamp support assembly. It shall be readily removable without removing the luminaire from the bracket arm.

The luminaire shall be attached to the bracket arm by means of a bracket entry and lamp support assembly. The assembly shall include a side entry slipfitter designed for 2" pipe with provision for 3° adjustment for leveling the luminaire. An enclosed terminal block shall be included such that all electrical connections shall be protected from exposure to weather.

All electrical connections shall be made waterproof or be made inside a weather resistant enclosure. All luminaires shall be ANSI/IES light distribution as indicated in plans. Each luminaire shall be labeled with a permanent label which states the type of lamp, voltage input, power input, power factor, ballast type, socket position, ANSI/IES light distribution, and such other catalog information that a complete replacement can be readily ordered.

The contractors attention is directed to those plan sheets detailing the mounting of luminaires at the pole top. Particular attention is directed to alignment of luminaire light distributions. Special attention must be exercised in the physical alignment of these luminaires to ensure that the approved photometric layout is physically produced at each lighting standard in the field. A marking shall be placed on the external face of the refractor to allow visual inspection of alignment. The marking shall correspond to the O° axis of the refractor.

FOOTING

The high mast foundations shall be constructed in accordance with the details shown in the plans.

Anchor bolts per manufacturer's Specifications. Submittals shall be supplied to the engineer of record prior to purchase.

One leveling nut, one hold-down nut, and one locking/jam nut shall be supplied per anchor bolt. All small metal parts, (nuts, screws, washers, etc.) shall be rustproofed either by galvanizing per ASTM Al53 or by the nature of the material used in their fabrication.

LOWERING SYSTEM SPECIFICATIONS

The lowering system shall consist of the following:

- A. Head frame and cover
- B. Luminaire ring C. Cables
- D. Winch
- E. Portable power unit (| per project)

The head frame unit shall rigidly mate the top of the pole to the head frame platform. The platform with its associated sheaves, etc. shall be covered and raintight. The head frame structure shall be zinc coated steel, attached to the pole by means of a steel slipfitter. Head frame shall encompass six 5" nominal steel cable sheaves grooved to the exact cable diameter, for 180° cable bearing surface. The sheave shall be zinc electroplated to ASTM 164 and dipped in yellow chromate for corrosion resistance. Bearings and cable keepers shall have permanent lubrication. Three (3) stainless steel 7 x 19 aircraft cables of $\frac{2}{3}$ " or greater diameter shall be provided.

The pole cable shall be attached to the luminaire ring with a waterproof connector capable of withstanding the pull of the weight of the pole cable. Where the wire ropes are required to bend over sheaves or over the winch drum, the maximum working stress in the outer fibers of wire rope shall not exceed 20 % of the wire rope manufacturer's rated ultimate stress.

Drum design shall cause level wind of wire rope. The power cord shall travel on sheave (s) or a combination of rollers providing a radius for the cord of 6" or larger. Each end of the sheave (s) or rollers shall have a keeper to prevent the cable from jumping out of the roller track.

The head frame shall also include three (3) latching devices to support the luminaire ring assembly when the lowering device is not in operation. The latches shall be actuated by alternate raising and lowering of the hoisting cables. Locking of luminaire ring shall be signaled by indicators visible from ground. All moving parts of the latch mechanism shall be serviceable from the ground. Each of the three latches shall be strong enough, by itself, to support twice the weight of the ring and all the luminaires. Latching mechanisms which depend primarily upon spring operation or contain dissimilar metals are not acceptable. The latching mechanism shall not require adjustment after the original installation.

The luminaire ring shall be constructed of a minimum of 6" x 2" x 7 guage steel channel galvanized in accordance with ASTM Al23 Class "B" steel channel with the appropriate number of 2" steel pipe mounting arms. The luminaire ring shall be prewired with Type "W" or specially reinforced Type "S0" power cable with suitable conductor quantity and size for proper operation and Type "ST" distribution wiring with insulation suitable for at least 105° C. All power cables should be attached to the aluminum weathertight wiring chamber with weathertight cable connectors. A 600 volt terminal block, completely prewired shall be included in the weathertight wiring chamber. A weathertight twistlock power inlet shall be provided on the luminaire ring to allow testing of the luminaire while in the lowered position. The power inlet shall face away from the pole for easy access.

The ultimate support of the luminaire ring shall not be dependent upon the lowering and raising cables.

The system shall be provided with circuit-breaker switches and twistlock disconnects in the pole base. Raising speed of luminaire ring shall be a minimum of 12' per minute.

The winch shall be a reversible worm gear self locking type with an integral friction drag brake to prevent freespooling. The winch shall be designated for hand operation or for operation by means of a $\frac{1}{2}$ " heavy duty reversing electric drill motor, remote controlled to enable the operator to stand 25' from the pole, Stainless Steel 7 x /9 aircraft cables of $\frac{1}{4}$ " or greater diameter equal to MIL-W-5424 shall be supplied on the winch. The winch shall be provided with keepers above the drum to force the cable away from the ends of the drum for spooling. The drum shall have a wire guard to prevent the cable from coming off.

The winch shall be mounted in such a way that the cable terminator and the riser cable connector may be reached and worked on by a person with his arm through the handhole.

Roller contact spring-loaded centering arms shall be provided to center the luminaire ring while ascending or descending the pole. The rollers for the centering arm shall be made of a water resistant non-marking composition material. All shafts and washers shall be #304 stainless steel. The spring-loading mechanism shall consist of an oil-tempered steel compression spring over an aluminum rod. The rollers shall be in contact with the pole at all times.

POLE SPECIFICATIONS

The pole shaft may be jointed or single piece, polygon or round, high strength steel having a minimum yield strength of 50 ksi. All material shall be single thickness steel plate with no laminations. Steel shall be as specified.

All poles shall be equipped with a reinforced handhole approximately l'above the base plate. The handhole shall be 10'' wide by 20'' high minimum.

All poles and hardware will be adequately packed to assure protection to the finish during shipping and handling, poles shall not be shipped pre-assembled.

Drawings shall be provided with the equipment which show assembly sequence, lift point, and recommended erection procedure. A permanent decal or card shall be fixed on the inside of the handhole cover which describes the sequence for lowering the luminaires and the cautions.

The proportioning of weld details and the operation of welding shall be in accordance with the current edition of the AASHTO Standard Specifications for Welding of Structural Steel Highway Bridges, and The Referenced American Welding Society Structural Welding Code.

Shop drill two (2) $\frac{\pi}{8}$ " diameter holes 180 degrees apart through total thickness of base plate. Tap top of hole for $\frac{\pi}{8}$ " x $\frac{3}{4}$ " || NC stainless steel hex head bolt.

Finished poles shall have a protective coating of hot galvanizing applied in accordance with ASTM Al23.

Note: It is the responsibilty of the contractor to coordinate the anchor bolt design with foundation design.

ALTERNATE POLE

A spun high mast prestressed concrete pole listed on the Qualified Products List may be substituted for a steel pole with approved shop drawings and calculations. If the concrete pole is provided as a substitute for the steel pole, payment will be made under the items bid for steel poles and associated foundations and plan quantity of these items will be the basis for payment.

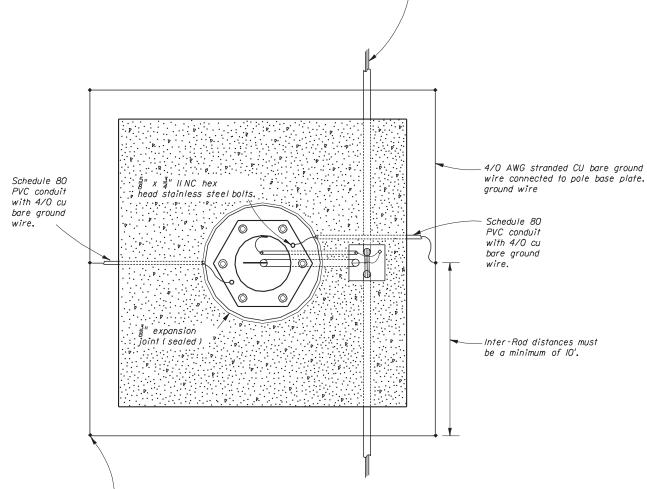
NOTES

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

HIGHMAST LIGHTING

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Drawn By			Revision	Sheet No.	Index No.
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I #6 AWG insulated (TW Green) stranded CU bond Wire connecting all poles, and insulated (THW or THWN) stranded copper circuit conductors in schedule 40 PVC conduit. Circuit conductors and conduit size as shown In plans. (Typical)

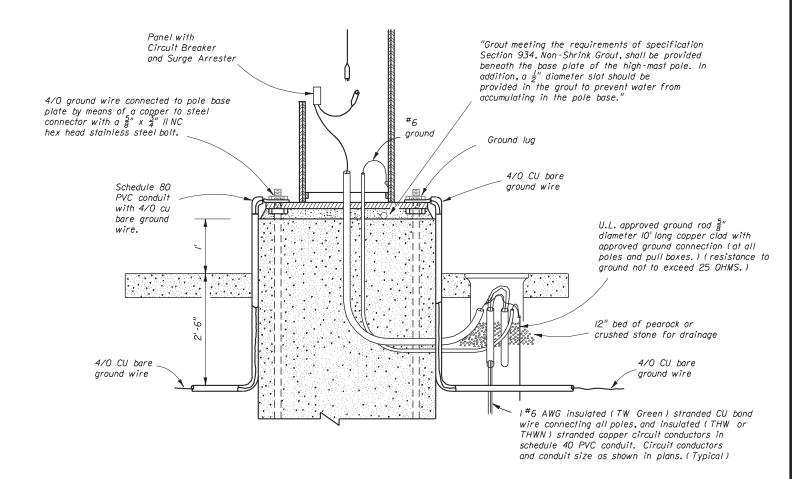


Minimum $\frac{8}{8}$ " x 20' approved ground rods (6). Maximum resistance to ground 25 ohms per rod and total system not to exceed 10 ohms.

Notes:

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- At all pull boxes and pole bases, ends of conduit shall be sealed in accordance with Section 630 of The Standard Specifications For Road And Bridge Construction.
- 2. I# 6 AWG insulated (TW Green) stranded CU bond wire connecting all poles, and insulated (THW or THWN) stranded copper circuit conductors in schedule 40 PVC conduit Circuit conductors and conduit size as shown in plans. (Typical)
- 3. Slabs to be placed around all Poles and Pull Boxes.
- 4. For Pull Boxes between Poles refer to index 17500 sheet 2 of 3.

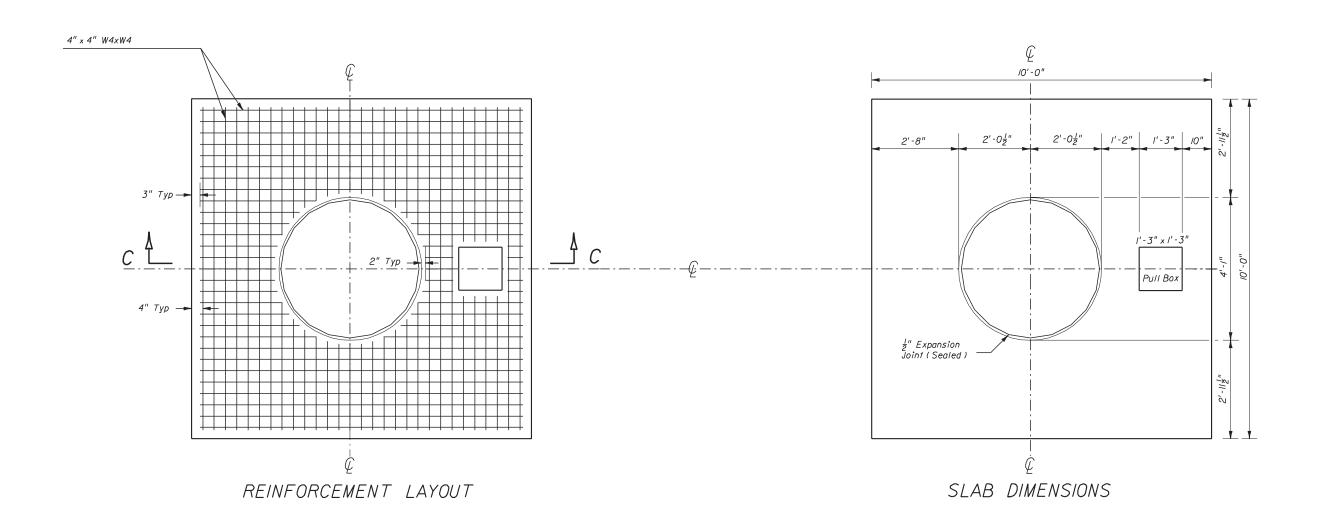


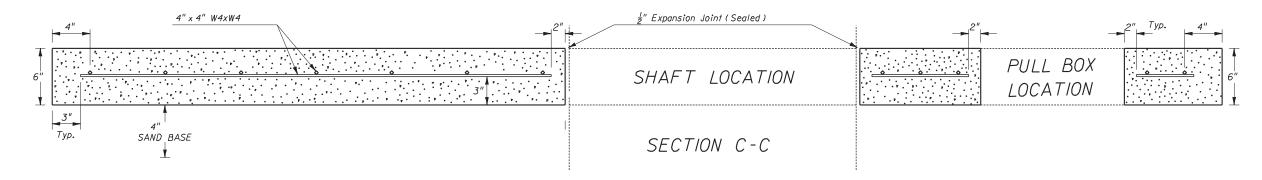
WIRING DETAILS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

HIGHMAST LIGHTING

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NOTES:

- I. Use clean free draining sand < 5% passing No. 200 sieve for base (4").
- 2. Welded wire fabric shall meet the requirements of ASTM A185.
- 3. Concrete strength at 28 days shall be f'c = 3 ksi.
- 4. Outside edges of slab shall be cast against formwork.

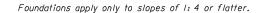
- 5. The $\frac{1}{2}$ " thick expansion joint between shaft and slab shall be sealed with a hot poured elastic joint sealer.
- Concrete slabs around poles and pull boxes shall be paid for under the contract unit price for Class I Concrete (Miscellaneous); the cost for reinforcing steel fabric shall be included in the price for Class I Concrete (Miscellaneous).
- 7. The pull box shown is I'-3" x I'-3"; others approved under Section 635 of the Standard Specifications may be used.

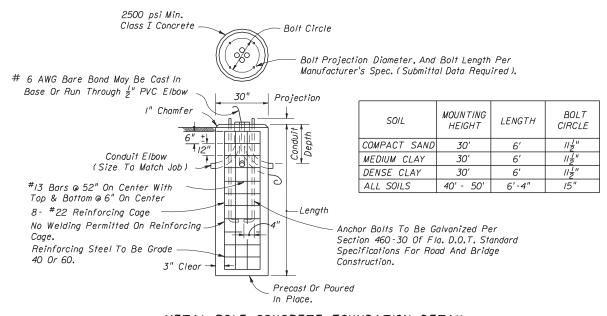
SLAB DETAILS

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

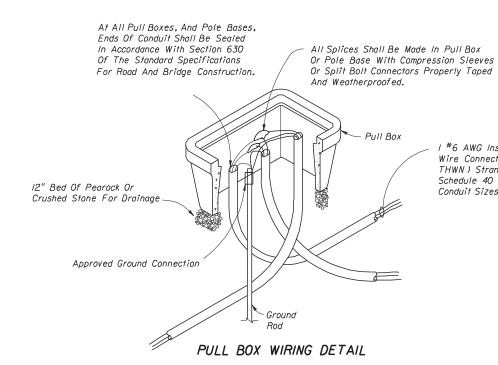
HIGHMAST LIGHTING

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METAL POLE CONCRETE FOUNDATION DETAIL



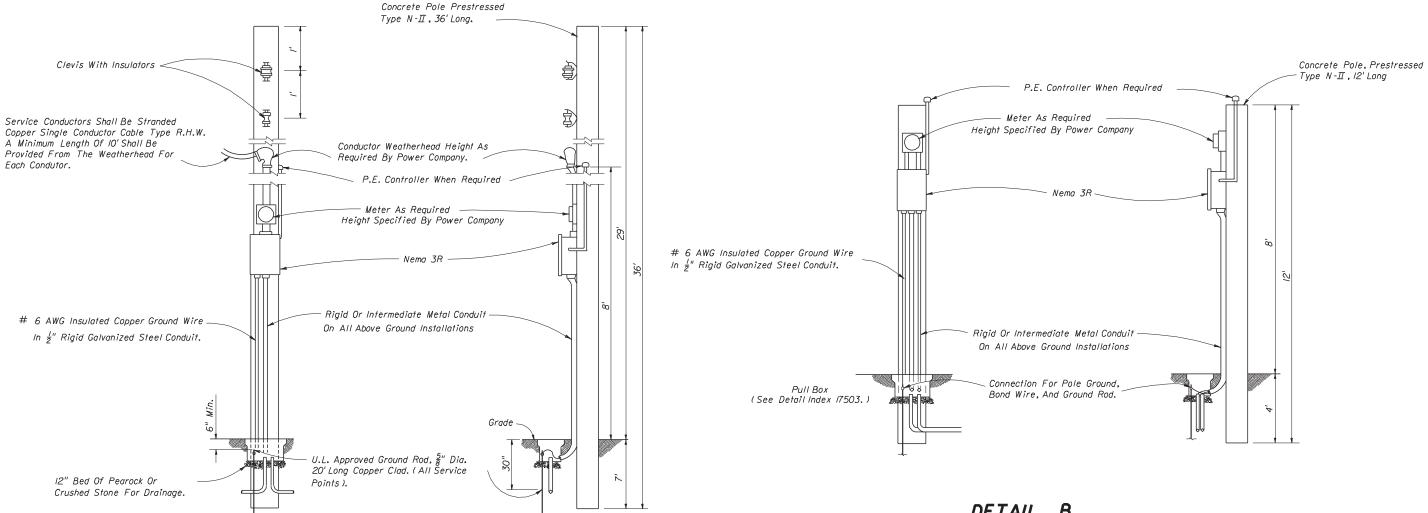
I #6 AWG Insulated (TW Green) Stranded CU Bond Wire Connecting All Poles, And Insulated (THW or THWN) Stranded Copper Circuit Conductors In Schedule 40 PVC Conduit. Circuit Conductors And Conduit Sizes As Shown In Plans. (Typical)

> STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

ROADWAY LIGHTING DETAILS

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NOTE : It shall be the contractors responsibility to provide a complete service assembly as per the plans and service specifications. The service installation shall meet the requirements of the national electric code and applicable local codes. Shop drawings are not required for service equipment, unless noted in the plans.



DETAIL A AERIAL FEED

Notes:

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- I. Photo electric control as required.
- 2. All neutral wires to have white insulation, do not use white or green insulated wires for ungrounded conductors.
- 3. A pull box is required at each service point.

I. The enclosure shall be NEMA 3R, pole mounted, rain-tight.

SERVICE SPECIFICATIONS

- 2. The enclosure door shall be lockable by padlock and four keys provided to the maintaining agency. The door shall have a minimum of three hinges and be latchable. No screws to be used to attach door.
- 3. 480 V minimum rating bolt-in type breakers shall be used.
- Busbar to be copper coated and have a minimum rating of IOO amps. When main breaker exceeds IOO amps busbar to match breaker amperage.
- 5. Locate contactor, transformer, and H.O.A. switch inside enclosure. The enclosure to be sized to accomodate as many breakers as called for and all other service equipment.
- 6. The Enclosure to be rigidly attached to the pole face.
- 7. A 600 V lightning protector shall be wired inside the enclosure.
- 8. A main breaker is required in all service panels with 2 or more feeder breakers.
- 9. All service equipment shall be U.L. approved.

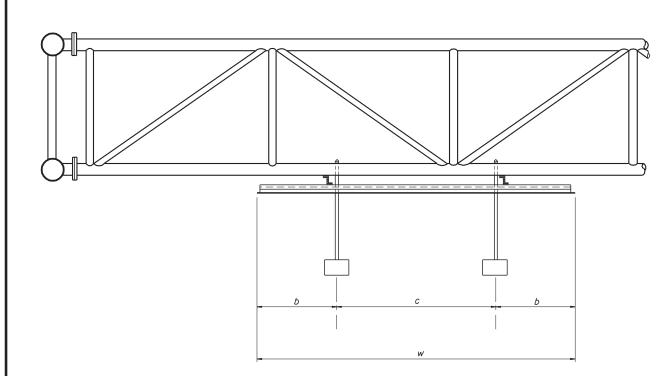
DETAIL B

UNDERGROUND FEED

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

SERVICE POINT DETAILS

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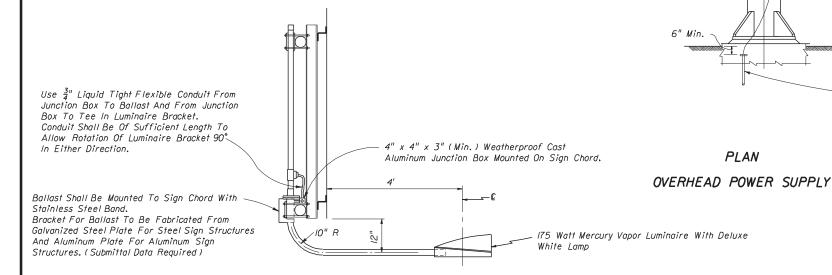


WIDTH OF SIGN FACE	To 10'	To 21'-6"	To 32'-6"	To 43'-4"
NUMBER OF FIXTURES	ONE	TWO	THREE	FOUR
EQUATIONS FOR PLACING FIXTURES	W = 2b	W = 2b + c	W = 2b + 2c	W = 2b + 3c
ALONG SIGN WIDTH	c = 0	c = 2.2b	c = 2.2b	c = 2.2b

PLACEMENT OF SIGN LIGHTS

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- I- Luminaire shall be mounted so the lamp center is 4' in front of the sign face.
- 2- Luminaire shall be mounted so the back of the fixture is placed I below the bottom edge of the sign face.
- 3- Luminaires from manufacturers who recommended their fixture be tilted shall be mounted on a bracket which provides this recommended tilt.
- 4- Photometric data for mercury vapor luminaire proposed for sign lighting shall be submitted for approval to the District Lighting Engineer, Florida Department Of Transportation.



SIGN LIGHTING INSTALLATION

Roadway Lighting included in contract:

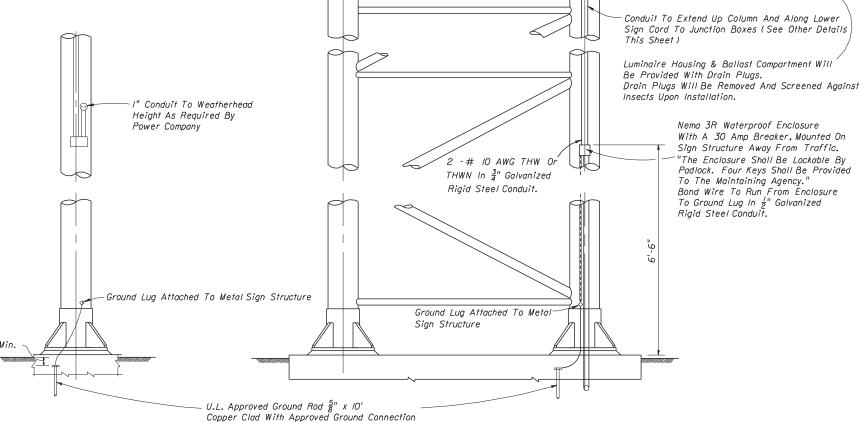
The power for the sign lighting shall be provided from the roadway lighting circuit. The lighting plans shall indicate the sign location and a pull-box location for connection to the sign lights. The lighting contractor shall install pull-box and loop 2' of lighting circuit conductors in the pull-box for connection by the signing contractor

The signing contractor shall furnish and install luminaires, Nema 3R enclosure, 30 amp breaker, conduit, conductors and all other electrical equipment necessary for connection to the lighting circuit.

Roadway Lighting not included in contract:

PLAN

The signing plans shall include pay item numbers to furnish and install conduit, conductors, ground rods, pull-boxes and service point equipment. The signing plans shall indicate the location of the service point equipment and circuit runs. The signing contractor shall provide all electrical equipment necessary for connection of the sign lights.



To Be Placed In Pull Box For Inspection Purposes. Resistance To Ground Not To Exceed 25 OHMS.

Splices To Be Made With Compression Sleeves Then Properly Insulated & waterproofed

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

For Details Of Luminaire Mounting Bracket

175 Watt Mercury Vapor Luminaire With Deluxe

White Lamp. Luminaire To Be Fused With A

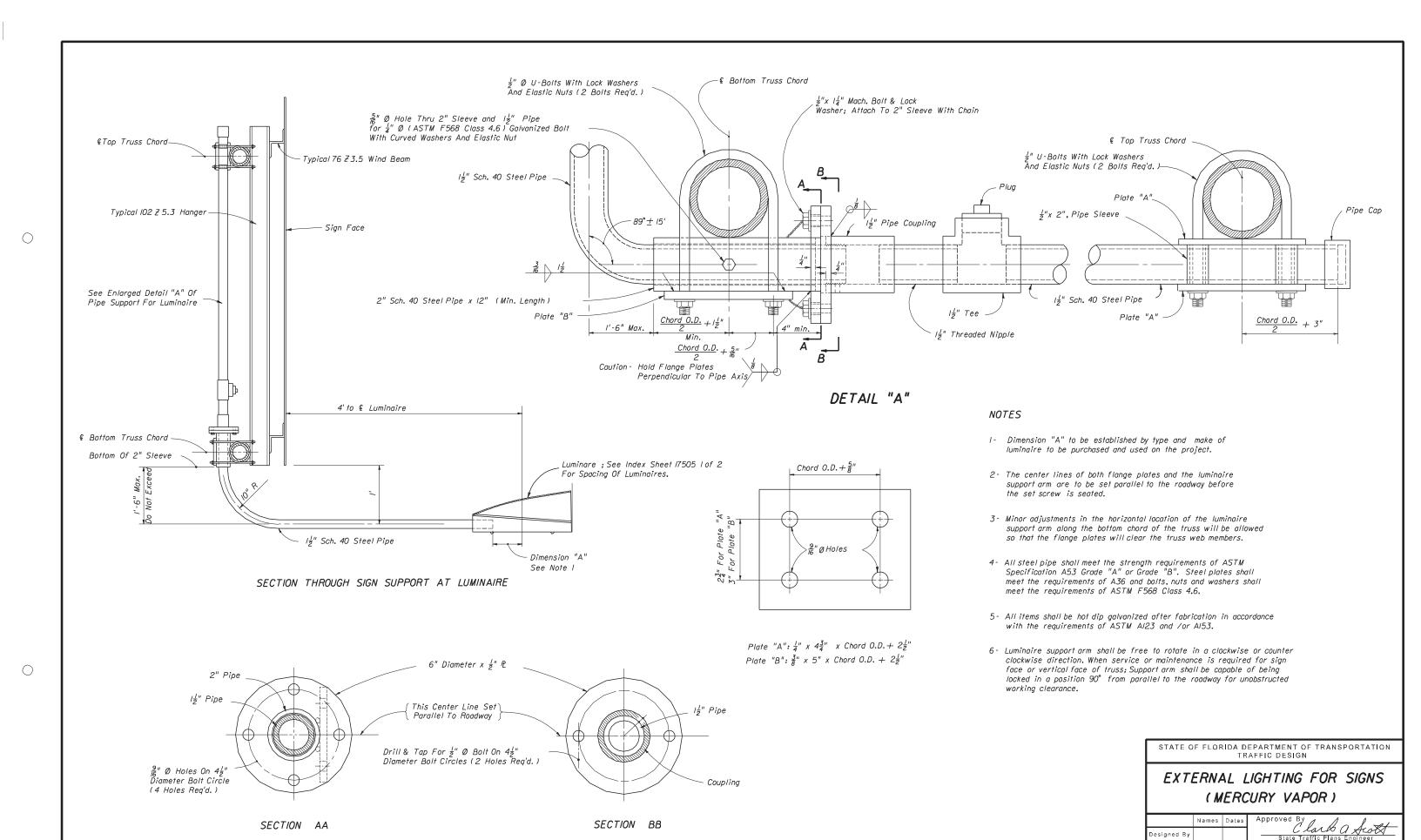
See Index 17505 2 Of 2

IO Amp FNQ Fuse._

EXTERNAL LIGHTING FOR SIGN (MERCURY VAPOR)

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SECTION AA

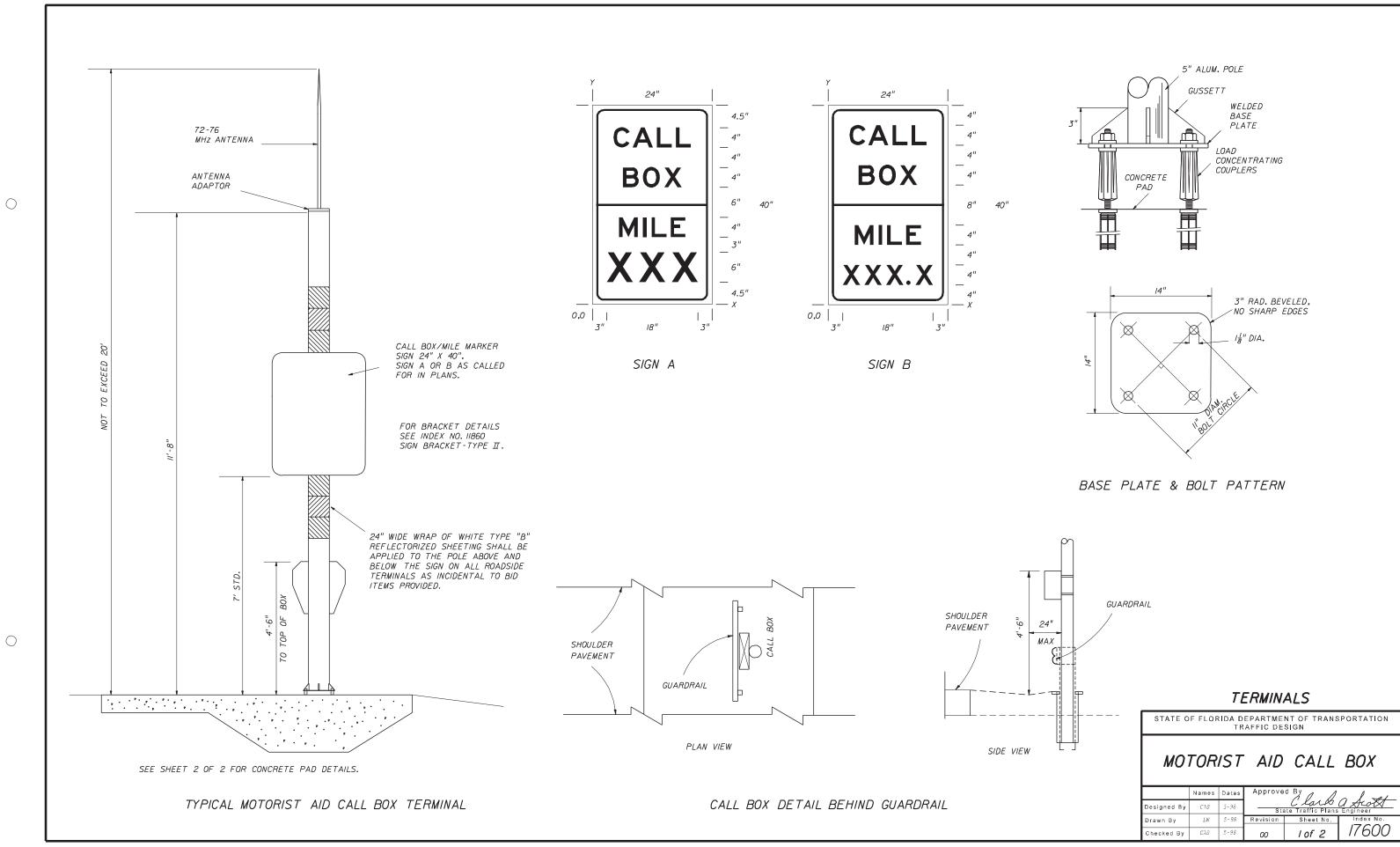
State Traffic Plans

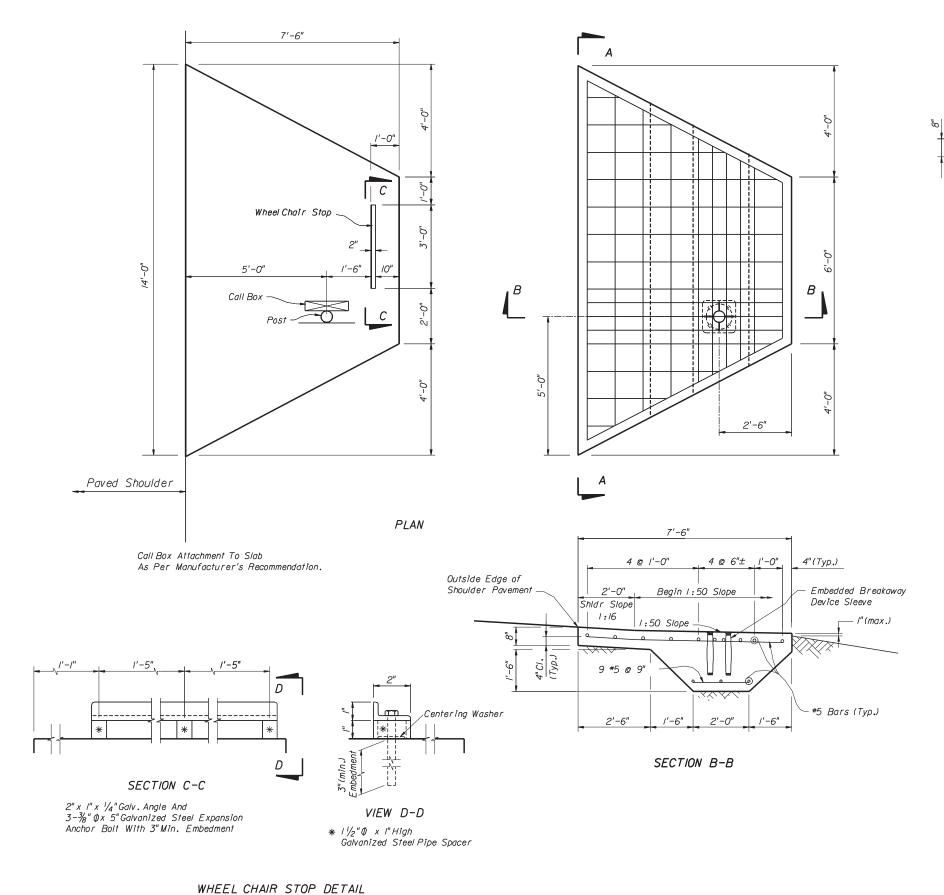
2 of 2

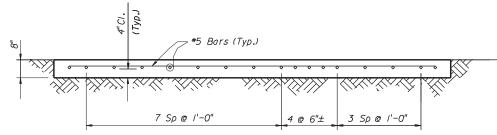
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Designed By

rawn By Checked By







SECTION A-A

MOTORIST AID CALL BOX CONCRETE PAD QUANTITIES

Concrete: 3.5 c.y. (each)
Reinforcing Steel: 243 lb (each)

GENERAL NOTES

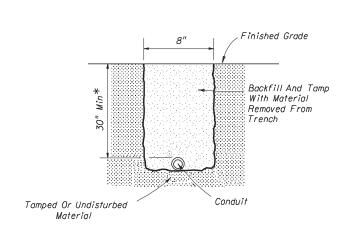
- I. General Specifications: FDOT Standard Specifications for Road and Bridge Construction (Current Edition) and Supplements thereto.
- 2. Design Specifications: AASHTO Standard Specifications For Highway Bridges (Current Edition and approved revisons thereto).
- 3. Concrete: Concrete strength shall be Class II (f'c=3,400 psi).
- 4. Reinforcing Steel: Reinforcing Steel shall conform to ASTM A615-96a, Grade 60.
- 5. Payment: Motorist Aid Call Box Concrete Pads shall be paid for under the contract unit price for Class II Concrete (Miscellaneous), c.y. and shall include all labor, materials, and installation of embedded breakaway device sleeves, and miscellaneous galvanized steel for wheel chair stop and attachments.
- 6. Breakaway Device shall be paid for under Call Box Assembly.

CONCRETE PAD

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

MOTORIST AID CALL BOX

	Names	Dates	Approve	d By /) 7	12/1
Designed By	TJR	4-98	State	Structures Des	ign Engineer
Drawn By	SHM	4-98	Revision	Sheet No.	Index No.
Checked By	TJB	4-98	00	2 of 2	17600

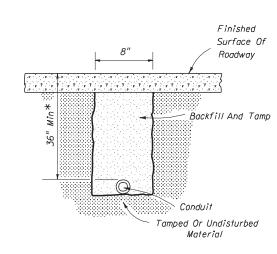


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FOR USE IN AREAS NOT EXPOSED TO VEHICULAR TRAFFIC AND UNDER DRIVEWAYS

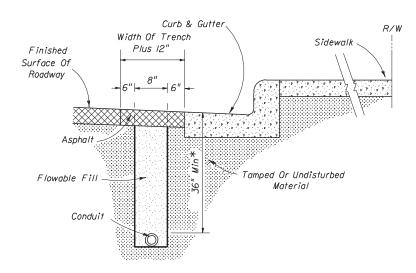
FIGURE A

*May be adjusted due to field conditions upon approval of project engineer.



FOR USE INSTALLING CONDUIT UNDER A NEW ROADWAY PRIOR TO INSTALLATION OF CURBS, BASE AND PAVEMENT

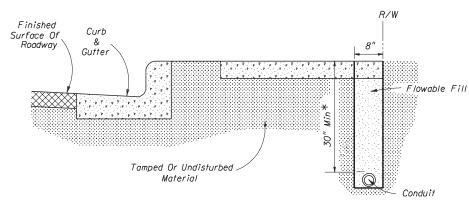
FIGURE D



FOR USE IN ASPHALT ROADWAY ADJACENT TO GUTTER WHEN PLACEMENT OUTSIDE OF THE PAVEMENT IS NOT FEASIBLE.

- I. Trench not to be open more than 3' at a time when construction area is subject to vehicular of pedestrian
- 2. Asphalt to be sawcut and removed to leave neat lines on both sides of the 12" pavement cut.
 3. See note 3 Figure C.

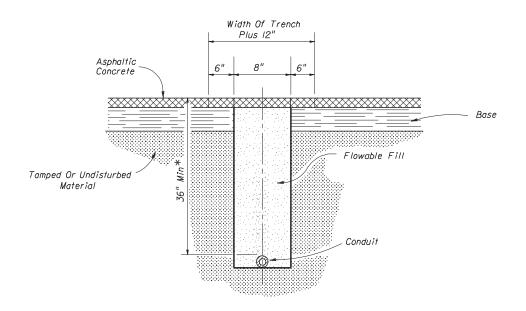
FIGURE B



FOR USE IN INSTALLING CONDUIT UNDER SIDEWALK

- I. Sidewalk patches to match existing joints.
- 2. Entire sidewalk slab must be replaced when specified in the plans.
- 3. Backfill and tamp with material from trench except at driveways. At driveways, backfill a length of trench within the driveway entirely with Flowable Fill.

FIGURE E



FOR USE IN INSTALLING CONDUIT UNDER EXISTING ASPHALT PAVEMENT NOT ADJACENT TO GUTTER WHEN JACKING IS NOT FEASIBLE

Note:

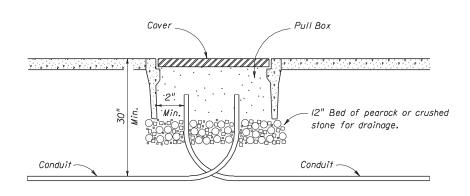
- I. Rigid conduit must be used when jacking under existing pavement at 36" minimum depth.
- 2. Asphalt to be sawcut at the edges of the trench.
- 3. The removal and replacement of the additional pavement width (6") will not be required when the trench can be constructed without disturbing the asphalt surface on either side.

FIGURE C

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CONDUIT INSTALLATION DETAILS

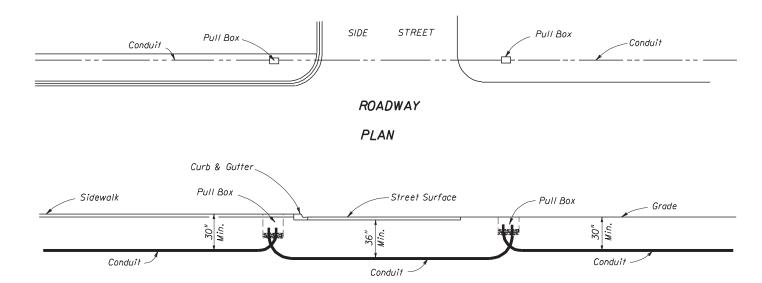
	Names	Dates	Approve	2 // //	
Designed By		2-75	Sta	C'LALAS ate Traffic Plans	
Drawn By			Revision	Sheet No.	Index No.
Checked By		2-75	00	1 of 2	17721



PULL BOX ENTRY OF CONDUIT UNDER SIDEWALKS

FIGURE A

Note: Ends of conduit shall be sealed in accordance with Section 630 of the Standard Specifications for Road and Bridge Construction.



UNDER SIDEWALK

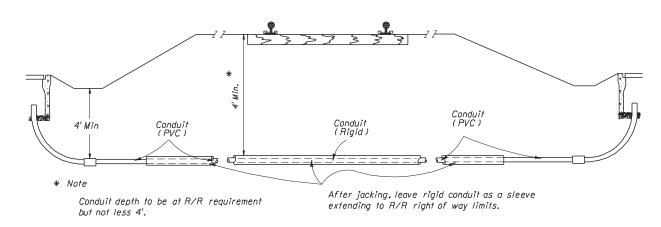
UNDER ROADWAY

UNDER NON-TRAFFIC BEARING SURFACE

SECTION

FIGURE B

Note:
One run of conduit (between pull boxes)
shall not contain more than 360° of bend
including pull box bends.



FOR USE UNDER RAILROADS

FIGURE C

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

CONDUIT INSTALLATIONS **DETAILS**

	Names	Dates	Approve	d By	2 l. 44
Designed By			Sta	ate Traffic Plans	Acott Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	2 of 2	17721

NOTES:

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Design Poles (Concrete and Strain Poles) in accordance with the latest edition of the AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals" and Supplement thereto. For allowable unit stresses, meet the requirements of Section 6.

Place the prestressing symmetrically. Supply a sufficient amount of prestressing to provide a calculated compressive stress of 2.2 ksi forType N-II and 3 ksi forType N-III at the top of pole after all losses.

Concrete Strength shall be 6 ksi minimum at 28 days and 4 ksi minimum at transfer of the Prestressing force.

Reinforcing steel shall be A6/5 Grade 60. Provide a minimum area of non-prestressed reinforcement equal to 0.33% of the concrete area.

Prestressed Strands shall be A416 Grade 270 stress relieved or low relaxation.

One turn required for spiral splices and two turns required at the top and bottom of poles. Spiral shall be manufactured from cold-drawn steel wire meeting the requirements of ASTM A82.

Attach span wire assemblies (consisting of the catenary wire, the messenger wire, and the tether wire) to the concrete poles in accordance with Section 634.

If a two point attachment is required by the plans, provide an eye bolt hole for the messenger wire, or field drill one at the location indicated in the plans. Field drill the eyebolt hole for the tether wire, when required, prior to

Use cover plates made of non-corrosive materials and attached to the pole using lead anchors or threaded inserts embedded in the pole and round head chrome

Attach ground wires to the reinforcing steel in the pole as necessary to prevent the ground wire from being displaced during concreting operations.

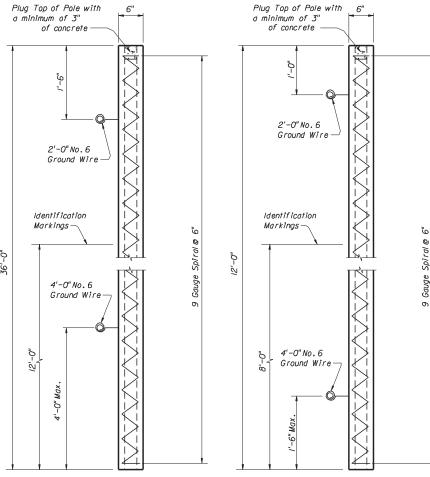
Identify concrete poles as to pole manufacturer, Department's pole type, length and Qualified Product List qualification number by inset numerals I" in height inscribed on the same face of the pole as the handhole and ground wire.

Provide a Class 3 Surface Finish as Specified in 400-15.2.4.

Provide a minimum cover of I".

Provide all poles with total taper of 0.152 IN/FT.

Plug Top of Pole with 11/2" D Galvanized a minimum of 3" of concrete Threaded Nipple -O"No.6 Ground Wire Identification Markings .3" x .5" Handhole. with Cover of No.6 Ground Wire

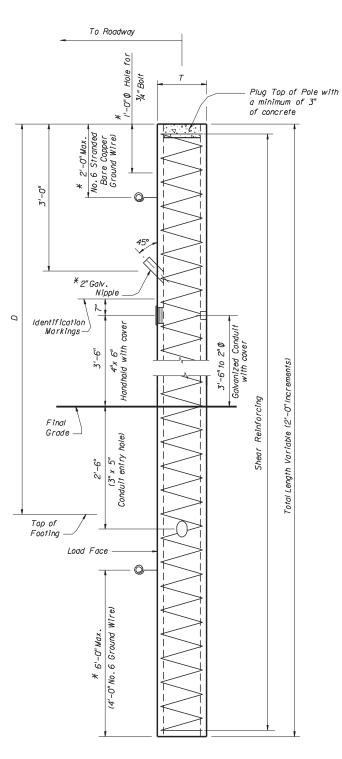


TYPE N-II POLE ON CONCRETE PEDESTAL

- 2"Galv. Nipple

SERVICE POLES - TYPE N-II

(For Installation, refer to Roadway and Traffic Design Standard, Index No. 17504)



POLE TYPES N-III THROUGH N-VII

* Do not apply these Items to Type N-III. Establish bolt hole locations, ground wire location and conduit location as shown in the plans.

**M	**MINIMUM REQUIRED MOMENT CAPACITY						
D (feet)	TYPE OF POLE						
D (reerr	N-IV $(k-ft)$	N- <u>√</u> (k-ft)	$N-\underline{VI}(k-ft)$	N- <u>∇</u>]] (k-ft)			
20	33	106	152	200			
22	37	111	159	218			
24	41	116	163	226			
26	44	121	172	234			
28	48	127	179	242			
30	52	132	185	250			
32	56	137	192	258			
34	60	142	199	266			
36	63	148	205	274			
38	67	153	212	282			
40	71	/58	2/9	290			
42	75	163	225	298			
44	79	<i>168</i>	232	306			
46	82	173	239	3/4			
48	86	177	245	322			
50	90	180	252	330			

Service Conditions: Design poles to carry the "Minimum Required Moment Capacity." These moments are based on a dead load plus wind load combinations, therefore obtain the allowable stresses by multiplying those for normal exposure conditions given in Section 6 by the applicable factor from Section 2 of the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals.

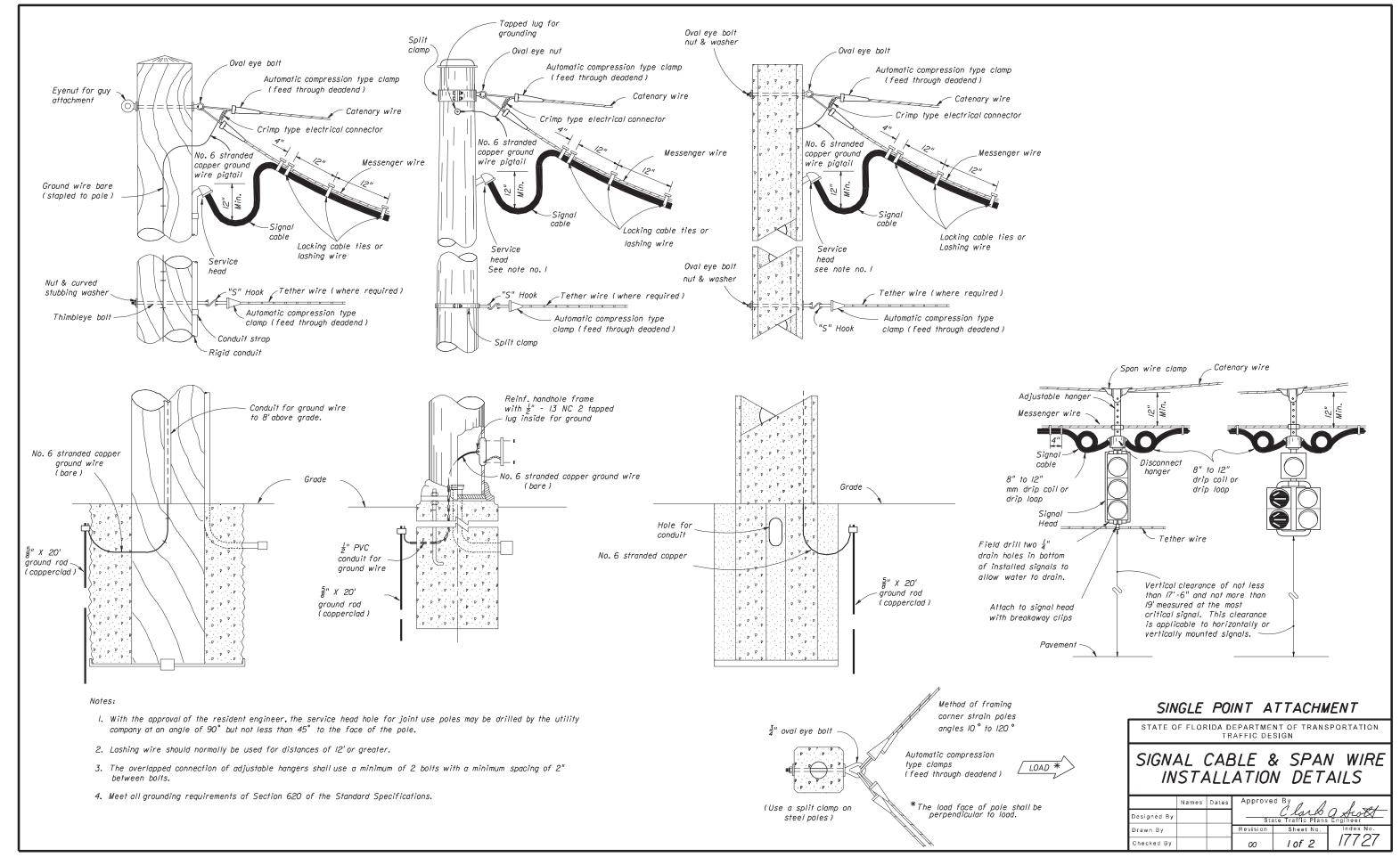
The ultimate moment capacity of each pole shall be a minimum of 1.3 times the "Minimum Required Moment Capacity."

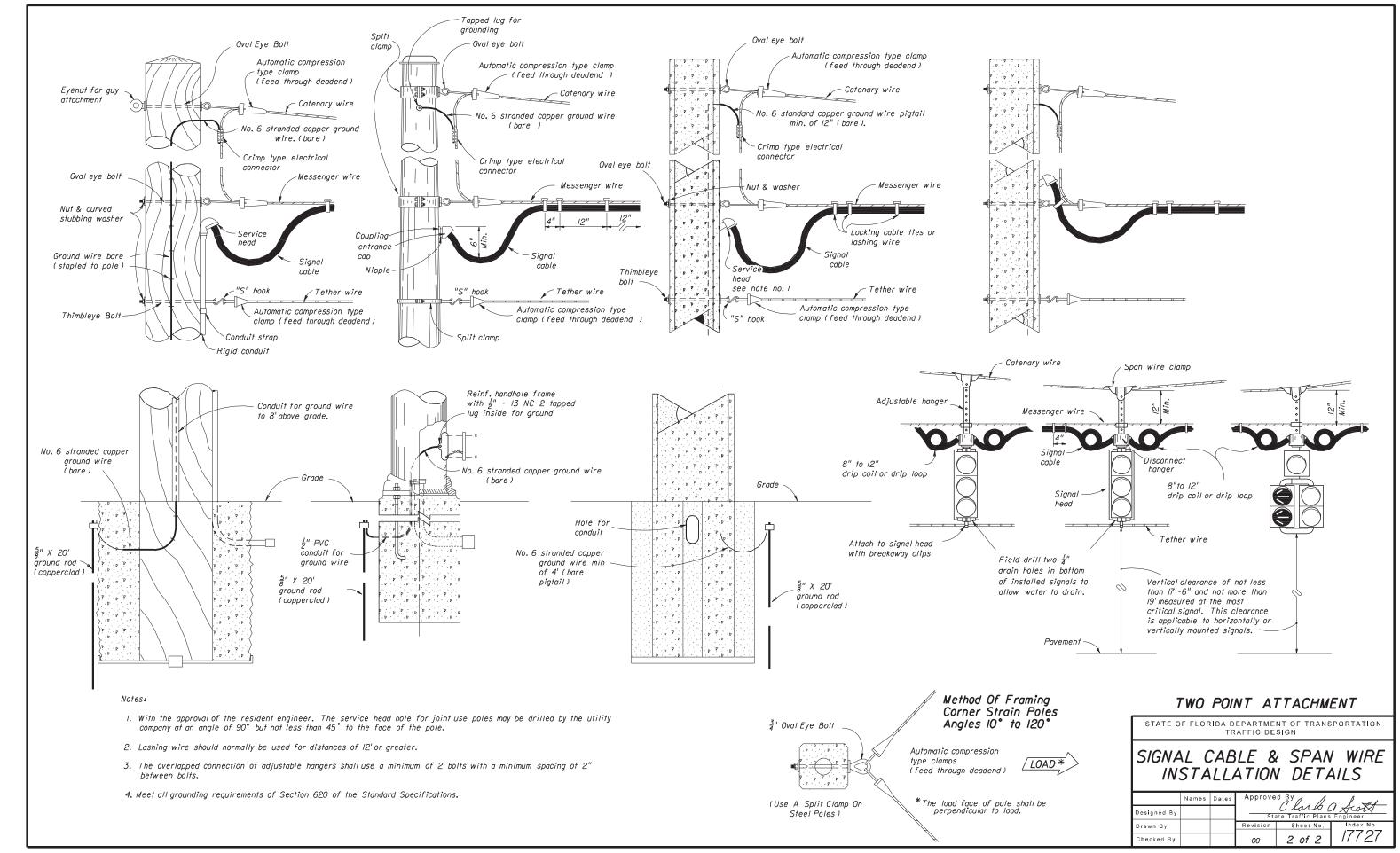
TYPE OF POLE	CONCRETE POLE				
	SIZE AT TOP (T)	SHEAR REINFORCING			
Type II	6" x 6"	9 Gauge Spiral@ 6"			
Type III	6" x 6"	6 Gauge Spiral @ 6"			
Туре <u>IV</u>	8" x 8"	5 Gauge Spîral@ 6"			
Туре 🏻	10" x 10"	5 Gauge Spiral @ 6"			
Type ☑ I	12" x 12"	5 Gauge Spiral © 6"			
Type ⊻ II	14" x 14"	5 Gauge Spiral@ 6"			

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CONCRETE POLES

	Names	Dates	Approve	d By	12/1
Designed By			Stat	te Structures De	sign Engineer
Drawn By	JP	10/99	Revision	Sheet No.	Index No.
Checked By	TJB	10/99	00	l of l	17725





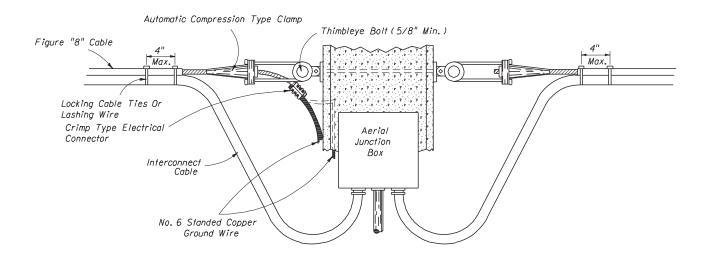


FIGURE A

CABLE DROP AND
TERMINATION DETAIL
AERIAL INTERCONNECT FIGURE "8"

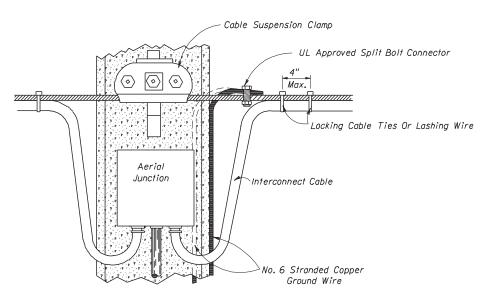


FIGURE C

CABLE DROP DETAIL
AERIAL INTERCONNECT MESSENGER
WIRE WITH CLAMPS

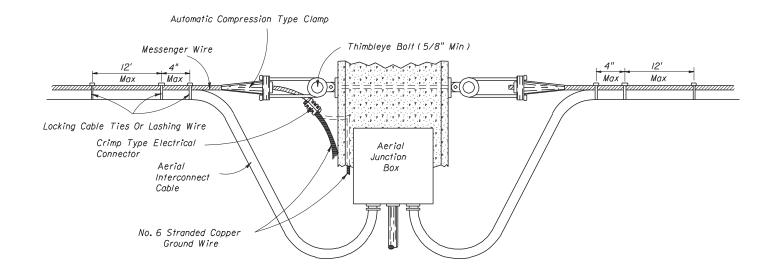


FIGURE B

CABLE DROP AND TERMINATION DETAIL AERIAL INTERCONNECT MESSENGER WIRE WITH CLAMPS

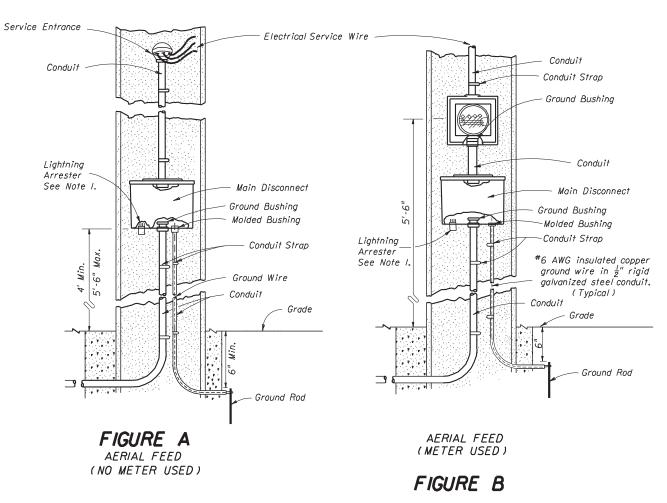
Notes:

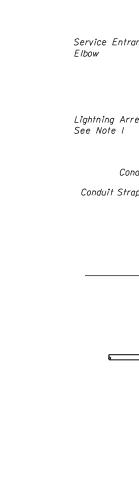
- The messenger wire of the interconnect cables shall be grounded to the copper ground wire of the pole or to the external wire extending down the pole.
- 2. When utilizing the external ground wire to the pole, a piece of $\frac{1}{2}$ " conduit shall extend up the pole externally to a point 8' above finish grade to protect the ground wire connecting the messenger wire to the ground rod.
- 3. Locking cable ties or lashing wire when used shall be placed no further than I2" apart except at the point of cable drop or terminations where one (I) shall be placed at the point where the cables separate from the messenger wire and another placed 4" (max) from that tie. When using figure "8" interconnect cable only the locking cable ties shall be used.
- 4. If accessible the internal ground wire of the support pole may be used to ground the messenger wire.
- 5. Lashing wire should normally be used for distances of 12' or greater.
- 6. Meet all grounding requirements of Section 620 of the Standard Specifications.

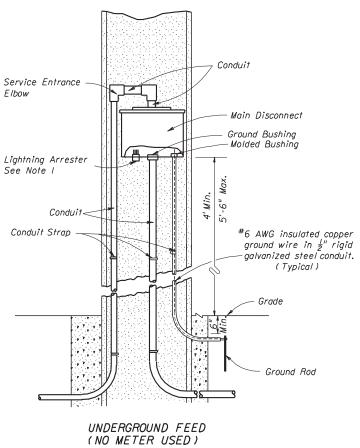
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

AERIAL INTERCONNECT

	Names	Dates	Approve	1 11	- 1 -1	
Designed By			State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	l of l	17733	







Conduit Service Entrance Elbow Grounding Bushing Main Disconnect Conduit Molded Bushing (Rigid, Galv.) Size 41 Min. Conduit Conduit Strap Lightning Arrester See Note I #6 AWG insulated copper ground wire in ½" rigid galvanized steel conduit. Grade . (Typical) Ground Rod Service TYPE "B" UNDERGROUND FEED (METER USED)

FIGURE D

FIGURE C

NOTES:

- I. The lightning arrester can be located on the side or bottom of the main disconnect enclosure at the Contractor's Option.
- 2. Liquidight flexible conduit is approved for use from the electrical disconnect to the cabinet when both are installed on the same pole.
- 3. Bond all elements together to form an Intersection Grounding Network in accordance with Section 620 of the Department's current Standard Specifications for Road and Bridge Construction. The bond wire shall be run in conduit with the Electrical Service Wire or Signal Cable.
- 4. Meet all grounding requirements of Section 620 of the Standard Specifications.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

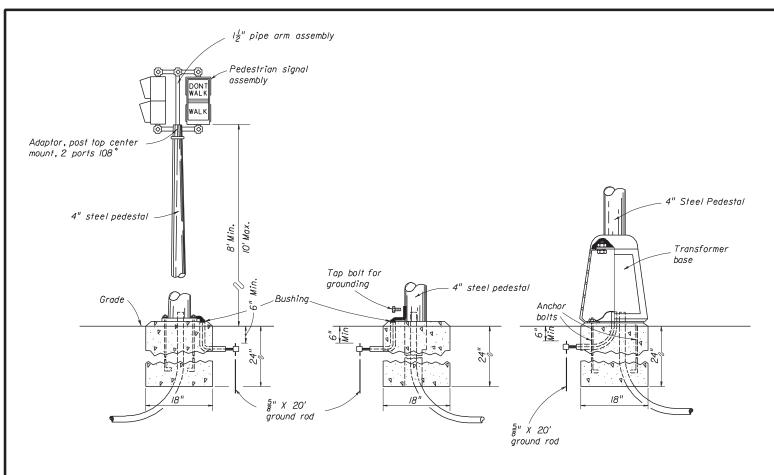
ELECTRIC POWER SERVICE

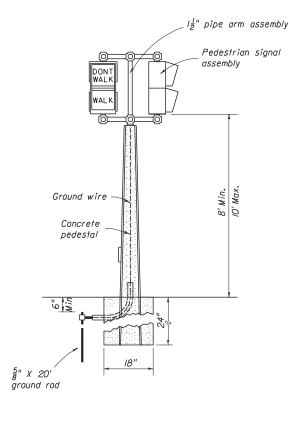
	Names	Dates	Approve			
Designed By		9-80	Clark a Acold State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	I of I	17736	

Molded Bushing (Non-Metallic) #6 AWG insulated copper ground wire in ½" rigid galvanized steel conduit. (Typical)	ster
Grade Grade Grade Ground Rod From Service From Service	

UNDERGROUND CABINET MOUNTED
(METER USED)

FIGURE E





Notes:

- As an option, the contractor will be allowed to install pedestrian signals on concrete poles and pedestals with the use of lead anchors (two bolts same size per hub.) in lieu of the standard steel bonds.
- Holes drilled or punched in metal poles or pedestals shall be thoroughly reamed, cleaned of all burrs and covered with two (2) coats of zinc rich paint as specified in the standard specifications for road and bridge, constuction. Grommets or bushings shall be installed in holes.
- 3. Meet all grounding requirements of Section 620 of the Standard Specifications.

FIGURE A

FIGURE B

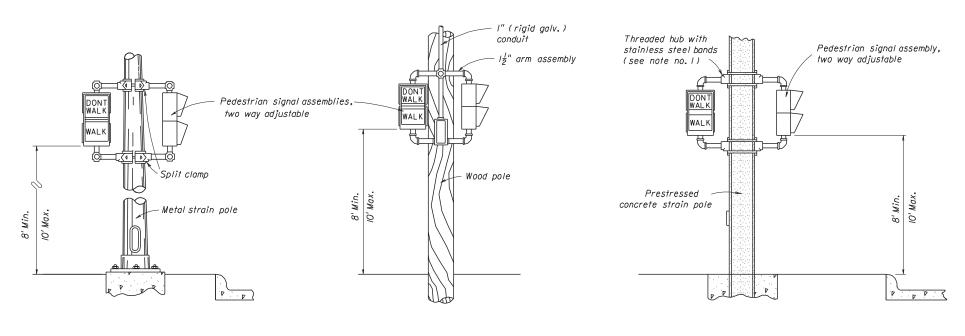


FIGURE C FIGURE D FIGURE E

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

PEDESTRIAN CONTROL SIGNAL INSTALLATION DETAILS

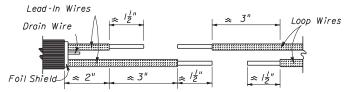
	Names	Dates	Approve	1 11 11	01 4	
Designed By		9-86	State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	I of I	17764	

DETAILS FOR SPLICING LOOP WIRE TO LEAD-IN WIRE

TWISTED PAIR AND LOOP LEAD-IN INSTALLATION WITH CURB & GUTTER

TWISTED PAIR AND LOOP LEAD-IN INSTALLATION WITHOUT CURB & GUTTER

STEP I

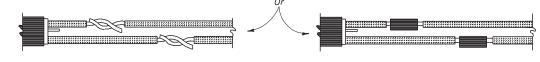


Strip Loop And Lead-In Cable Conductors, If Heat

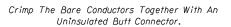
Shrinkable Silicone Lined, Cross Linked Polyethylene Insulating Tubing Is To Be Used, Slip Tubing Over Lead-In Cable And Individual Conductors.

STEP 2

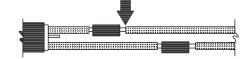
STEP 3



Twist The Bare Conductors Together.



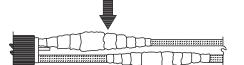




Solder Each Splice Using Resin-Core Solder.

Solder Each Splice Using Resin-Core Solder.

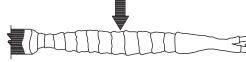




Wrap Each Splice With Silicone Tape. Half Lap Starting At Center Of Splice And Proceeding To The Right (Or Left) * 4" Past End Of Splice, Then Proceed ing To The Left (Or Right) $\approx \frac{3}{4}$ " Past Other End Of Splice And Returning To Center. Wrap Each Splice, With An All Weather Heavy Duty Electrical Tape In The Same Manner ≈ 3" Past Each End Of Silicone Tape.

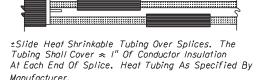


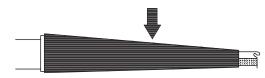
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Half Lap The Two Splices Together With An All Weather Heavy Duty Electrical Tape ≈ I" Past The End Of The Lead-In Cable Outside Cover And ≈ I" Past Farther Most Wrap Of Step 4.



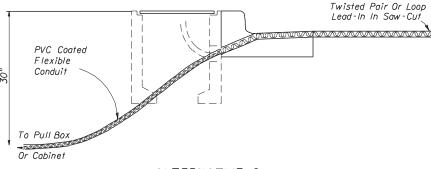




Slide Outer Heat Shrinkable Tubing Over Entire Splice Area. The Tubing Shall Cover ≈ l½" Of The Lead-In Cable Outside Cover And ≈ I" Of The Loop Conductor Insulation.

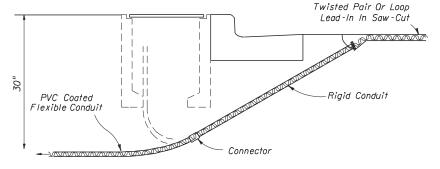
Drill A Hole Through The Curb At The Point Which The Required Saw-Cut Depth Is Obtained Just Prior To Cutting The Top Inside Edge Of The Curb. Slide A Section Of Flexible Conduit At Least 6" Into The Hole From The Back Side Of The Curb But Not Within 2" Of The Top Of The Hole. The Conduit Shall Fit Snug Within The Drilled Hole, Fill The Top Of The Hole With Loop Seglant To The Level Of The Curb Surface. A Nonmetallic Material Should Be Used To Prevent Excessive Loop Sealant From Entering The Flexible Conduit.

ALTERNATIVE I



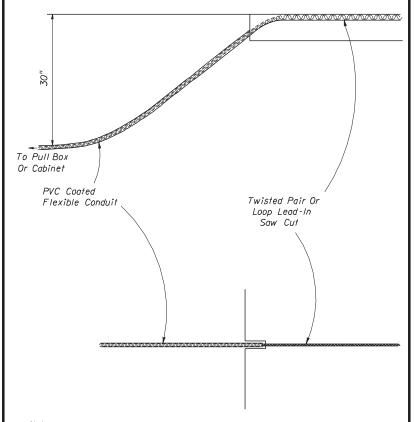
ALTERNATIVE 2

Drill A Hole 2" To I" Larger In Diameter Than The Rigid Conduit To Be Used Through The Roadway Asphalt (Or Concrete) Surface And Base At An Appropriate Angle To Intercept The Trench Or Pull Box Hole. Place A Predetermined Length Of Rigid Conduit In The Hole And Drive The Conduit Into The Trench Or Hole. Install A Molded Bushing (Nonmetallic) On The Roadway End Of The Rigid, Conduit. The Top Of The Rigid Conduit Shall Be Approximately 2" Below The Roadway Surface. Fill The Hole With Loop Sealant To The Level Of The Roadway Surface. A Nonmetallic Material Should Be Used To Prevent Excessive Loop Sealant From Entering The Rigid Conduit.



Other alternatives may be approved by the State Traffic Operations Engineer

Cut A Slot In The Edge Of The Roadway Of Sufficient Size And Depth To Snugly Place The End Of The Flexible Conduit. The End Of The Conduit Shall Be At Least 6" Into The Roadway And 2" ≈ Below The Top Of The Roadway Surface. The Departure Angle Of The Conduit From The Roadway Shall Be 30° To 45°.



Other alternatives may be approved by the State Traffic Operations Engineer

GENERAL NOTES

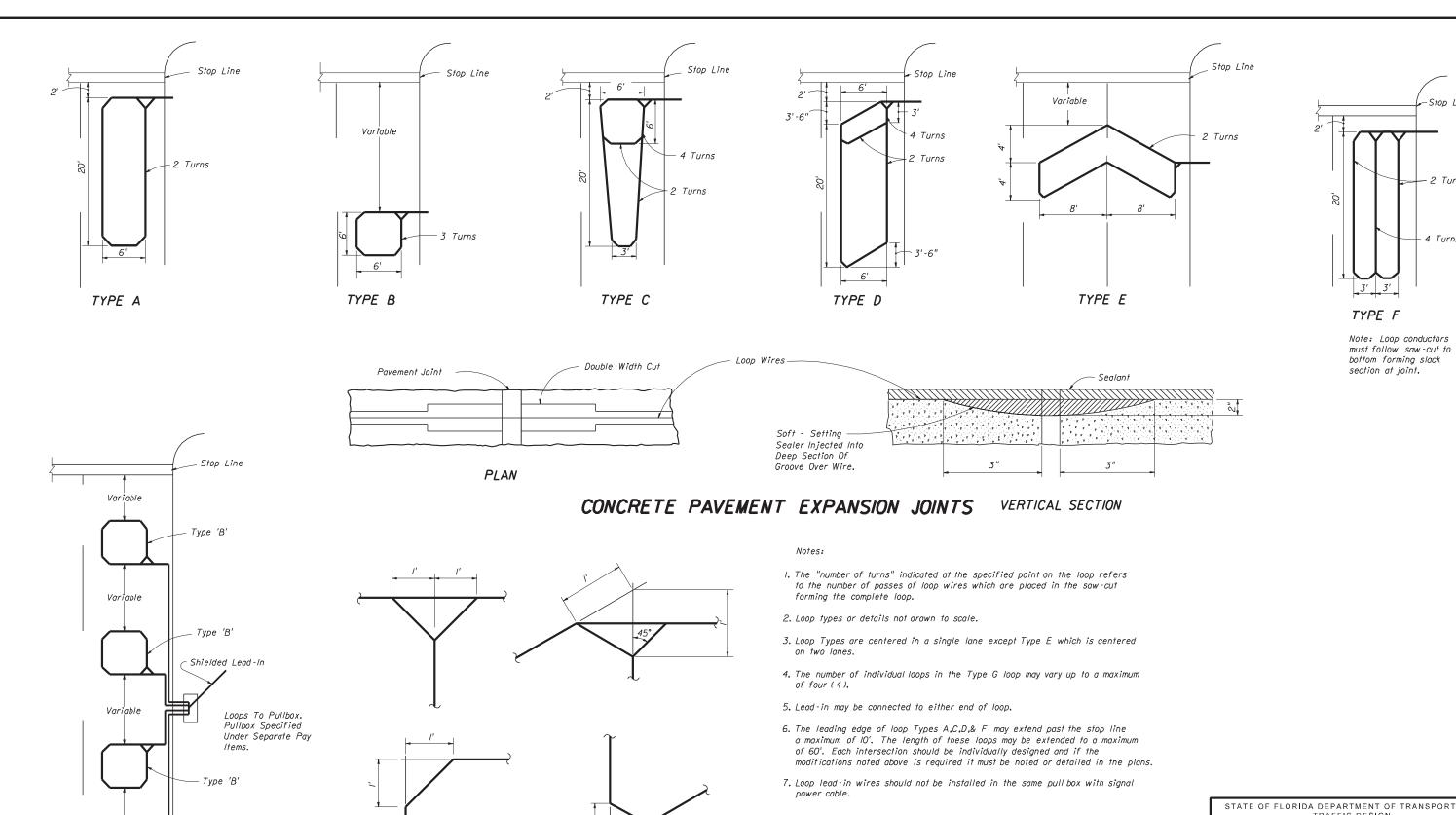
- I. If the loop lead-in is 75' or less from the edge of the loop to the detector or controller cabinet, continue the twisted pair to the cabinet. If the loop lead-in is greater than 75' continue the twisted pair to the specified pull box, splice to shielded lead-in wire and continue to the detector or con-
- 2. The width of all saw cuts shall be sufficient to allow unforced placement of loop wires or lead-in cables into the saw cut. The depth of all saw cuts, except across expansion joints, shall be 3" standard with a maximum of 4".
- 3. On resurfacing or new roadway construction projects, the loop wires and lead-in cables may be installed in the asphalt structural course prior to the placement of the final asphalt wearing course. The loop wires and lead-in cables shall be placed in a saw cut in the structural course. The depth of the cables below the top of the final surface shall comply with note 2.
- 4. A nonmetallic hold down material shall be used to secure loop wires and lead-ins to the bottom of saw-cuts. Hold down material shall be placed at approximately 12" intervals around loops and 24" intervals on lead-ins.
- 5. The minimum distance between the twisted pairs of loop lead-in wire is 6" from the loop to 12" from the pavement edge or curb.

6. Splice Connections in pull boxes may be made with U.L. listed, watertight, insulated connectors in lieu of the details above. The lead-in cable insulation shall be sealed using electrical tape or heat shrinkable tubing (refer to step 5 above). The seal shall extend approximately I" either side of the lead-in cable outer cover.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

VEHICLE LOOP INSTALLATION DETAILS

	Names	Dates	Approve		2 l AL	
Designed By			Clark a Scott State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No	
Checked By			00	I of 2	17781	



LOOP CORNER AND LEAD-IN DETAILS

Variable

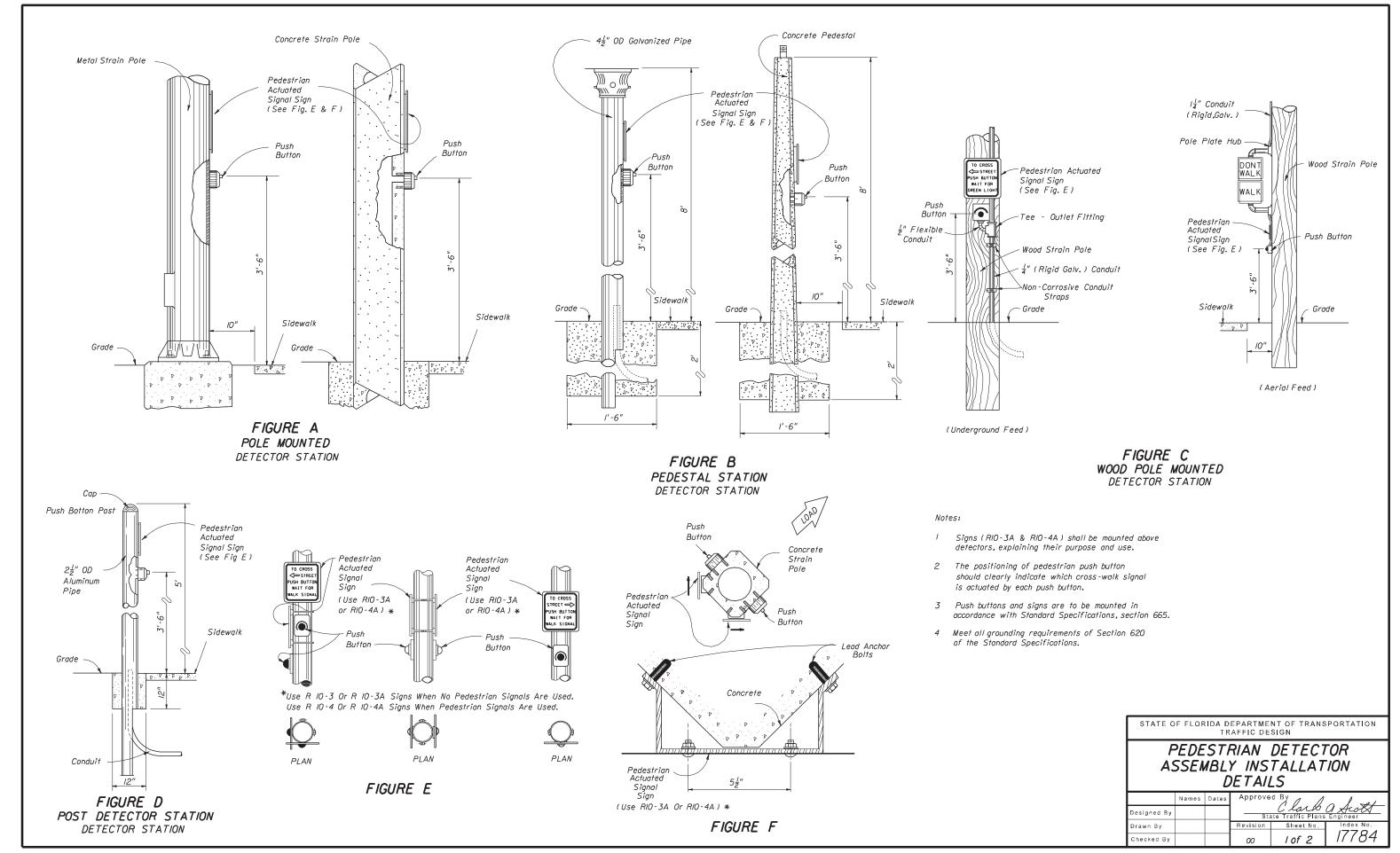
TYPE G

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

VEHICLE LOOP INSTALLATION DETAILS

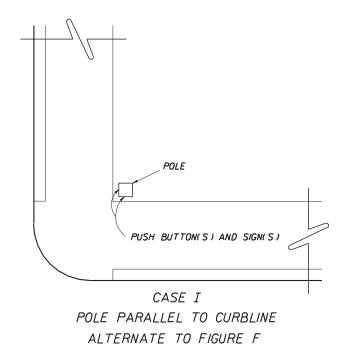
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Designed By			State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
Checked By			00	2 of 2	17781	

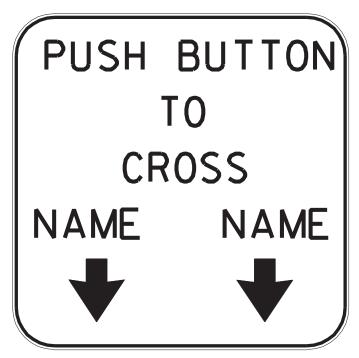
4 Turns



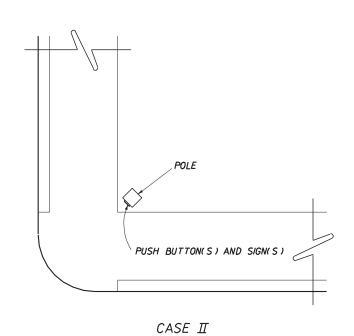
PUSH BUTTON TO CROSS ST NAME

FTP-47





FTP-48



POLE DIAGONAL TO CURBLINE

START CROSSING WALK WATCH FOR TURNING CARS STEADY DO NOT START DONT FINISH CROSSING WALK IF STARTED FLASHING DO NOT DONT ENTER WALK CROSSWALK STEADY

FTP-49

WHITE BACKGROUND WITH BLACK LEGEND AND BORDER

WALK PLAQUE - WHITE LEGEND ON BLACK BACKGROUND

DONT WALK PLAQUE - ORANGE LEGEND ON BLACK BACKGROUND

THE INTERNATIONAL SYMBOLS MAY BE USED FOR WALK AND

DONT WALK.

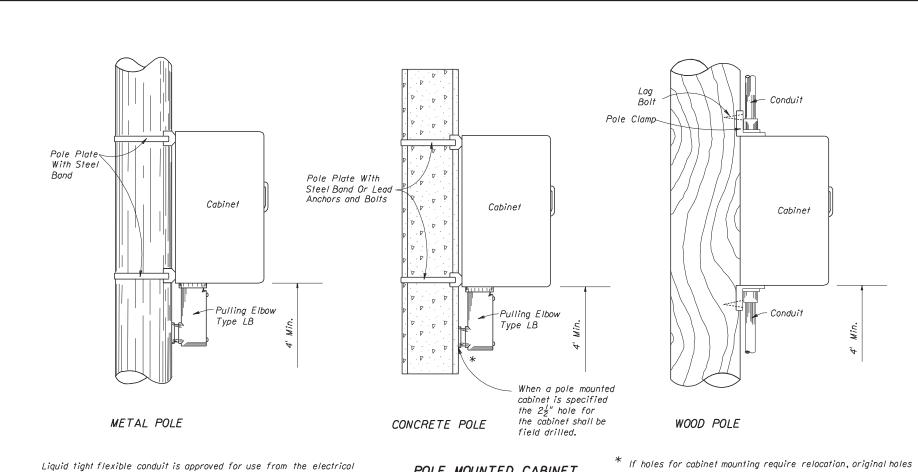
Note:

I. See Index 17355 for sign details.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

PEDESTRIAN DETECTOR

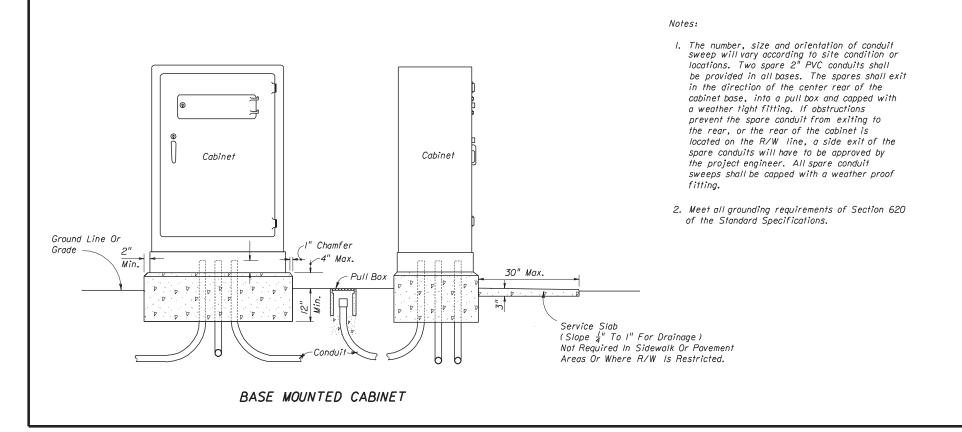
ASSEMBLY INSTALLATION
DETAILS



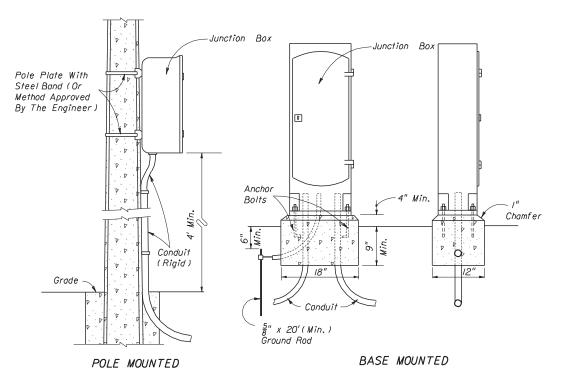
Liquid tight flexible conduit is approved for use from the electrical disconnect to the cabinet when both are installed on the same pole.

POLE MOUNTED CABINET

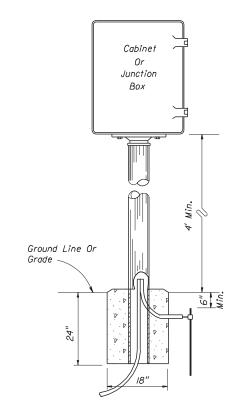
If holes for cabinet mounting require relocation, original holes shall be filled in with concrete or covered with a non corrosive cover plate.



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INTERCONNECT JUNCTION BOX

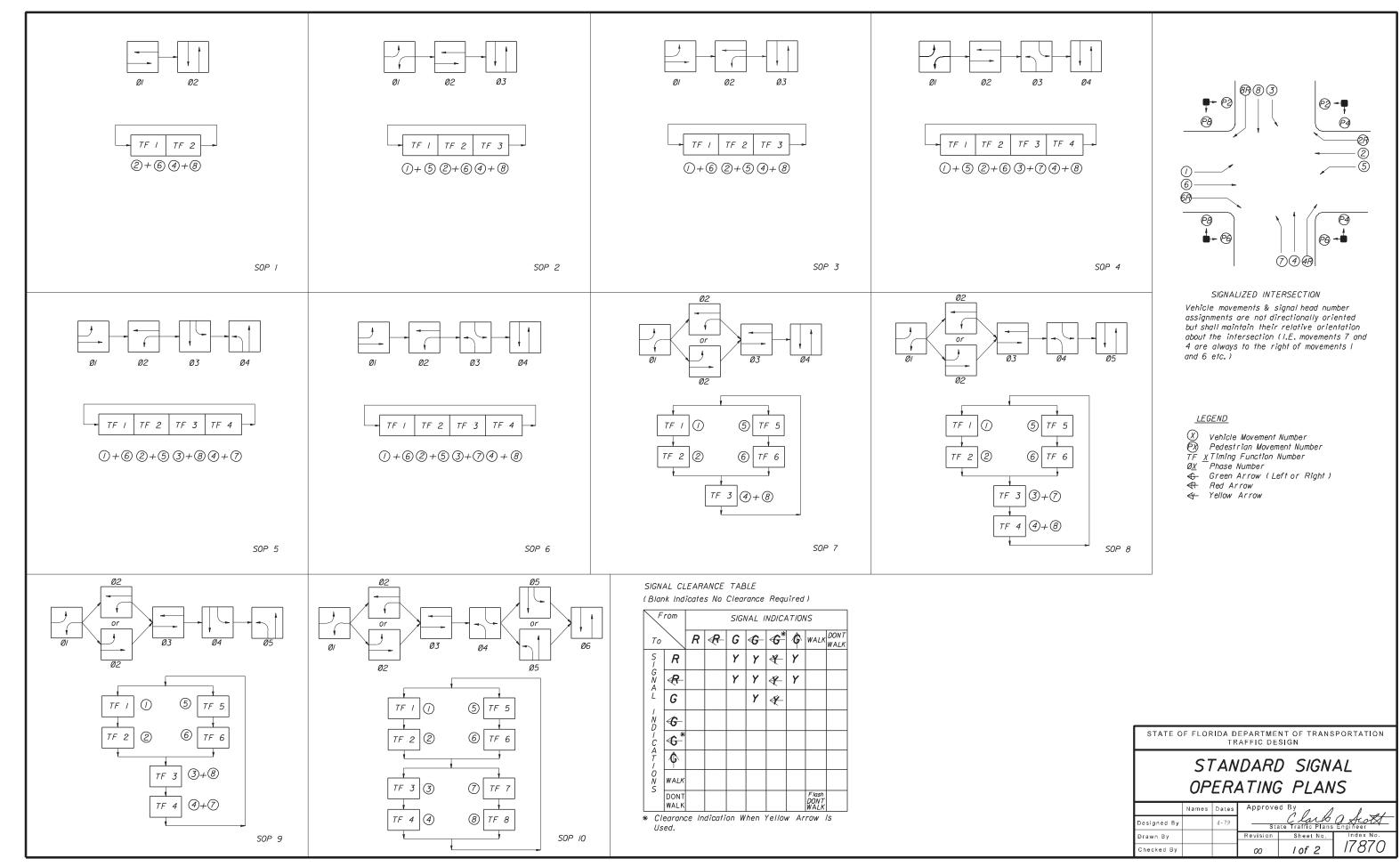


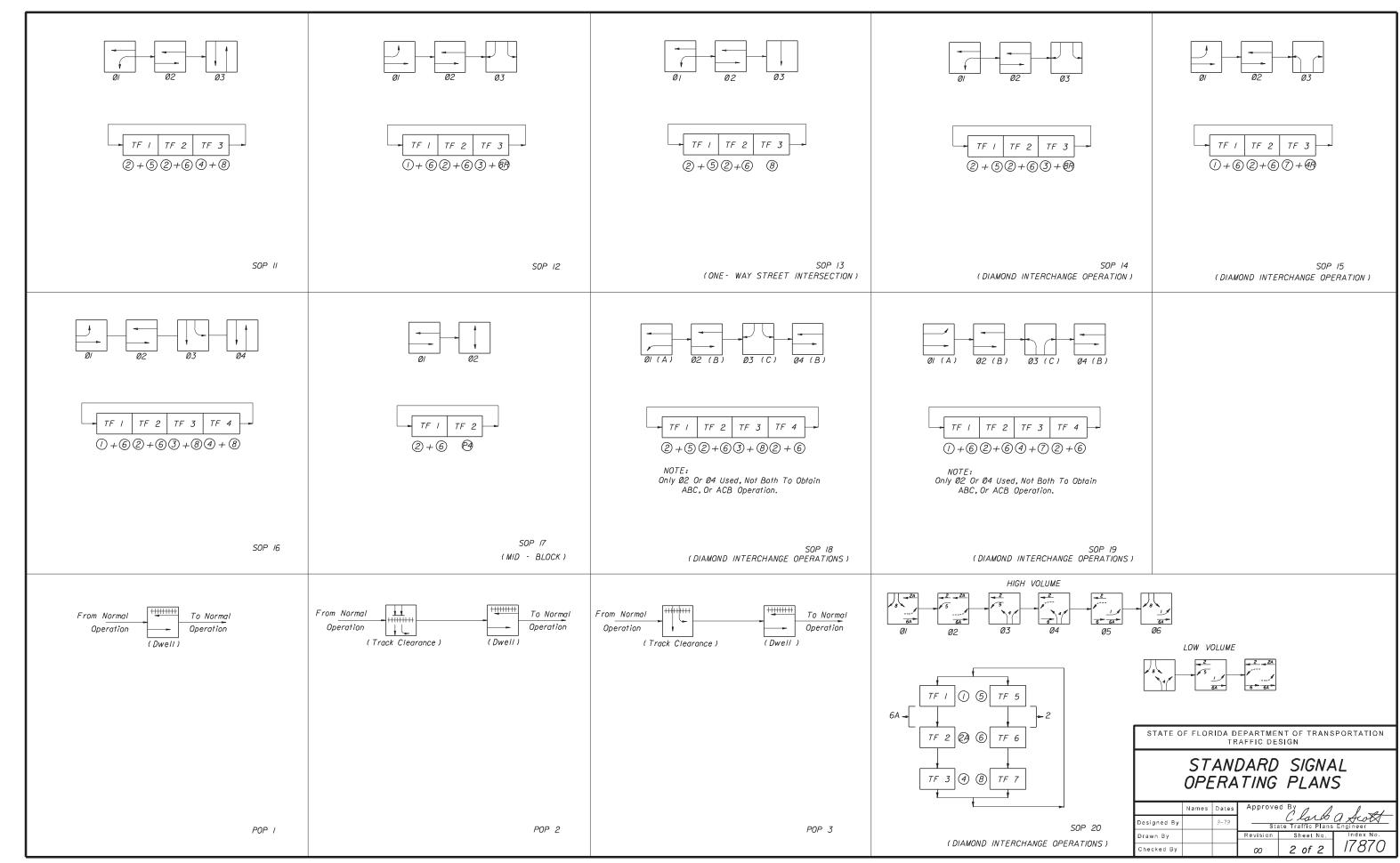
PEDESTAL MOUNTED

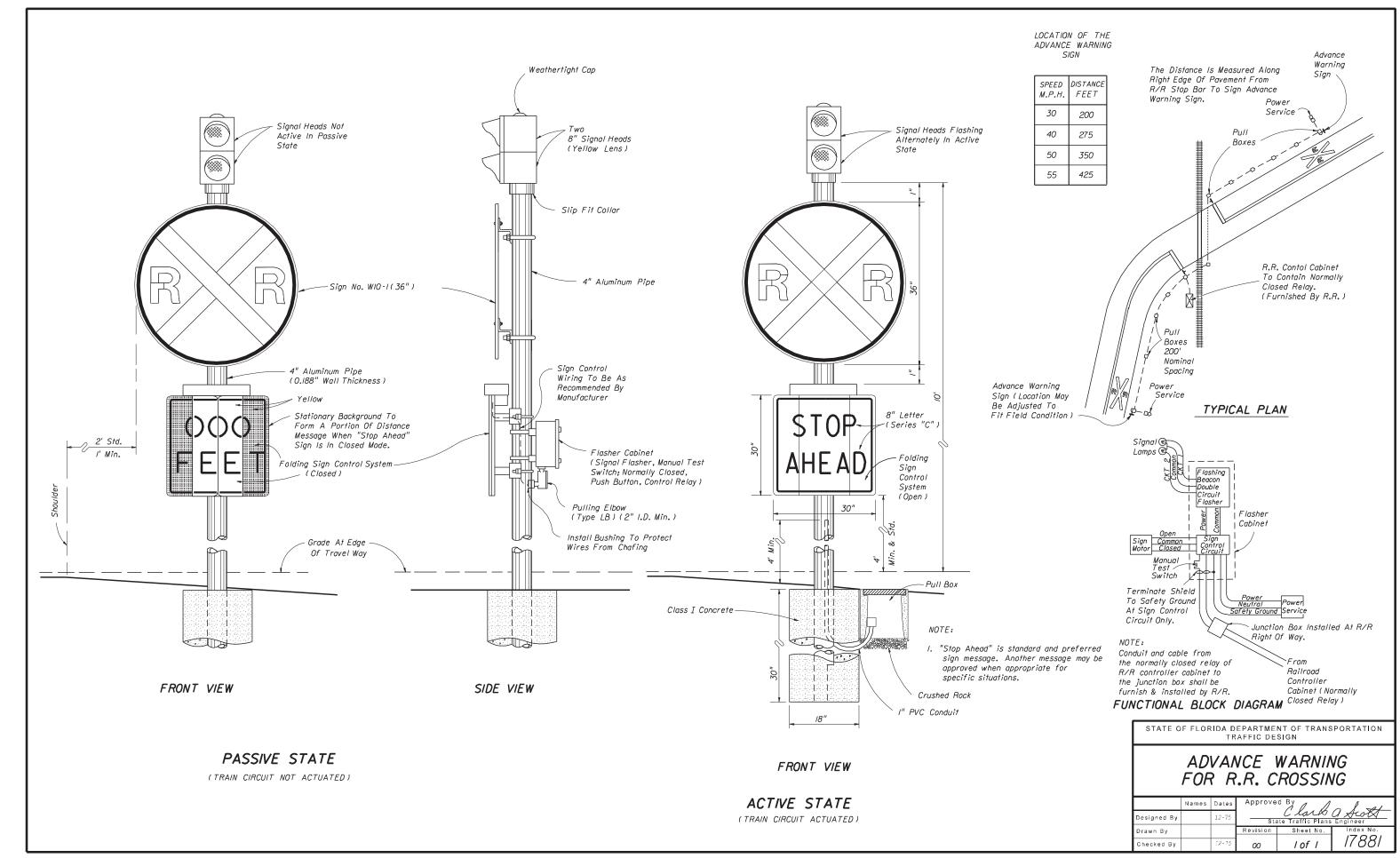
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

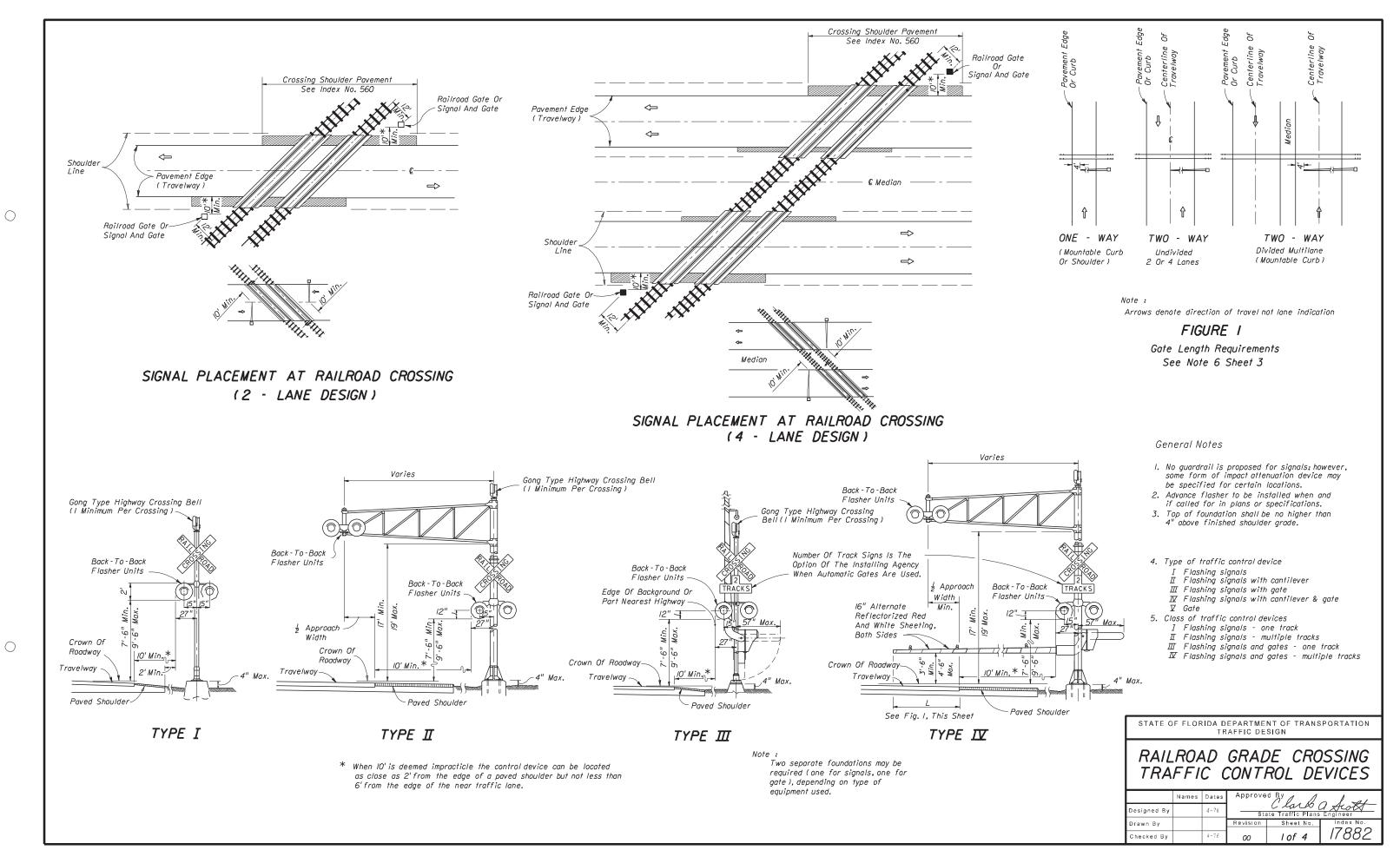
CABINET INSTALLATION DETAILS

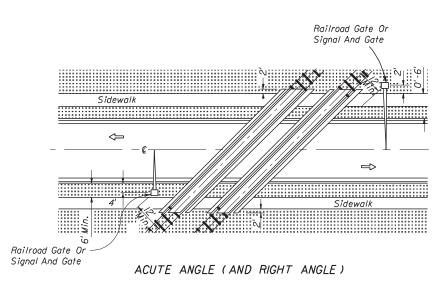
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Designed By			State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
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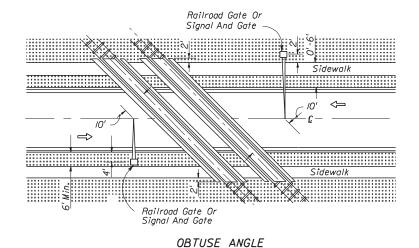








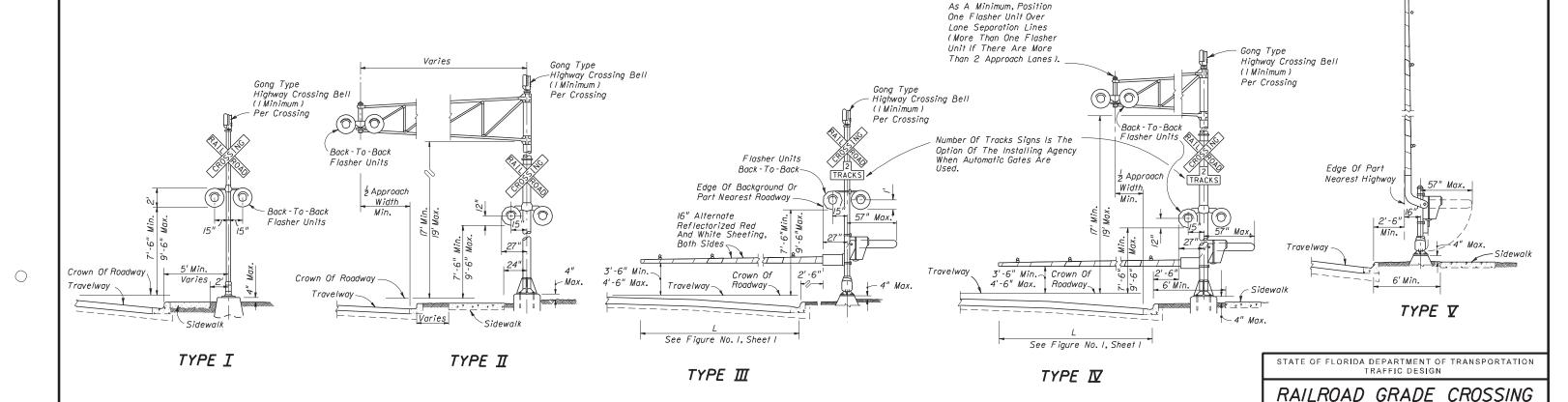
SIGNAL PLACEMENT AT RAILROAD CROSSING (2 LANES, CURB & GUTTER)



SIGNAL PLACEMENT AT RAILROAD CROSSING
(2 LANES, CURB & GUTTER)

GENERAL NOTES

- I. The location of flashing signals and stop lines shall be established based on future (or present) installation of gate with appropriate track clearances.
- 2.. Where plans call for railroad traffic control devices to be installed in curbed medians, the minimum median width shall be 12'-6".
- Location of railroad traffic control device is based on the distance available between face of curb & sidewalk.
 O' to 6' - Locate device outside sidewalk.
 Over 6' - Locate device between face of curb and sidewalk.
- 4. Stop line to be perpendicular to edge of roadway, approx. 15' from nearest rail; or 8' from and parallel to gate when present.



17882

State Traffic Plans

2 of 4

TRAFFIC CONTROL DEVICES

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Designed By

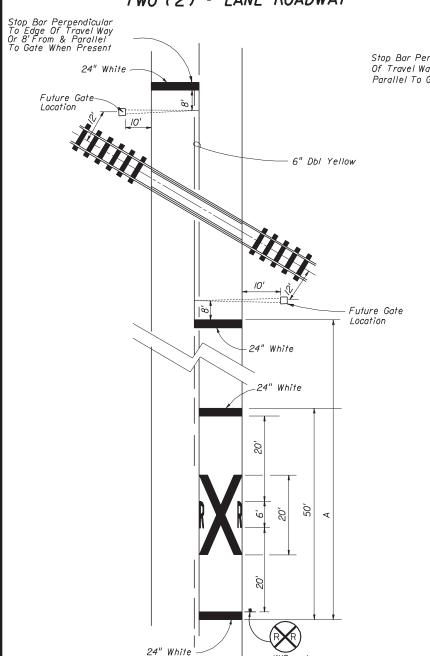
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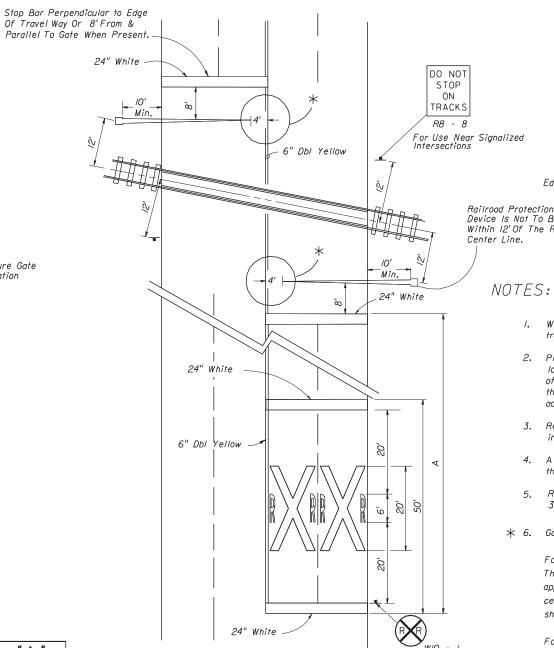
RAILROAD CROSSING AT TWO (2) - LANE ROADWAY

RAILROAD CROSSING AT MUTI-LANE ROADWAY

RELATIVE LOCATION OF CROSSING TRAFFIC CONTROL DEVICES



)		
SPEED MPH	" A " IN FT	
60	550	
55	450	
50	<i>3</i> 75	
45	300	
40	225	
35	150	
30	100	
URBAN	50 MIN.	



		With Gates	
Signalized	Stop Line	Flashing Signal (If Not With Gate)	
Ed Iroad Protection ice Is Not To B hin I2'Of The R ter Line.		As Required	
NOTES:			

Gate Or Flashing Signal

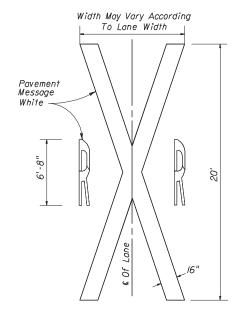
- When computing pavement message, quantities do not include transverse lines.
- Placement of sign WIO-I in a residential or business district, where
 low speeds are prevalent, the WIO-I sign may be placed a minimum distance
 of IOO' from the crossing. Where street intersections occur between
 the R/R pavement message and the tracks an additional WIO-I sign and
 additional pavement message should be used.
- Recommended location for sign FTP-38, IOO' Urban & 300' Rural in advance of the crossing.
- A portion of the pavement markings symbol should be directly opposite the WIO-I sign.
- 5. Recommended location for FTP-38 A or B signs, 100' urban and 300' rural. See index 17355 for sign details.
- * 6. Gate Length Requirements

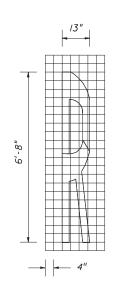
For two-way undivided sections:

The gate should extend to within I' of the center line. On multilane approaches the maximum gate length may not reach to within I' of the center line. For those cases, the distance from the gate to the center line shall be a maximum of 4'.

For one-way or divided sections:

The gate shall be of sufficient length such that the distance from the gate tip to the inside edge of pavement is a maximum of 4'.

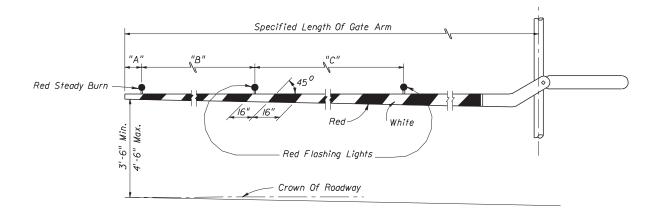




STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

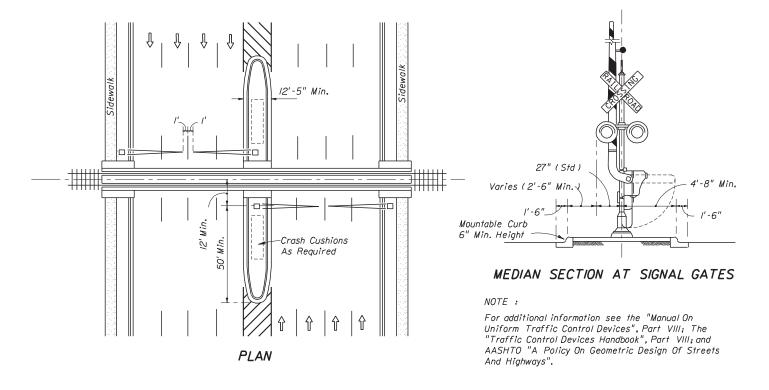
RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES

	Names	Dates	Approve	n' // //	a l 21
Designed By		10-77	State Traffic Plans		Engineer
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	3 of 4	17882



RAILROAD GATE ARM LIGHT SPACING

Specified Length	Dimension	Dimension	Dimension
Of Gate Arm	"A"	"B"	"C"
14 Ft. 15 Ft. 16-17 Ft. 18-19 Ft. 20-23 Ft. 24-28 Ft. 29-31 Ft. 32-34 Ft. 35-37 Ft. 38 And Over	6" 18" 24" 28" 28" 28" 36" 36" 36"	36" 36" 36" 4!" 4' 5' 6' 7' 9'	5' 5' 5' 5' 5' 6' 7' 9'



MEDIAN SIGNAL GATES FOR MULTI LANE UNDIVIDED URBAN SECTIONS

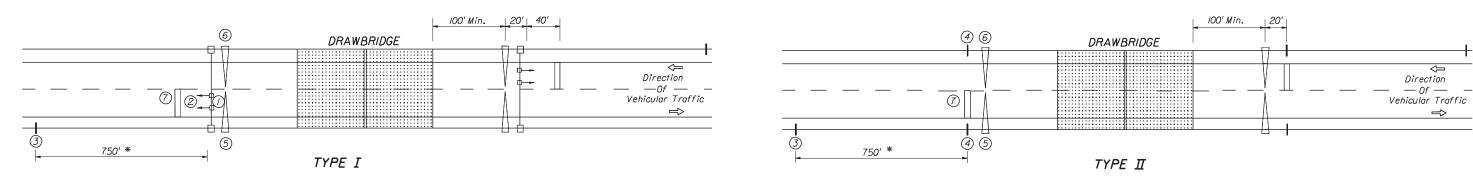
(THREE OR MORE DRIVING LANES IN ONE DIRECTION, 45 mph OR LESS)

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

RAILROAD GRADE CROSSING TRAFFIC CONTROL DEVICES

	Names	Dates	Approved By			
Designed By		10-85	Clark O Acold State Traffic Plans Engineer			
Drawn By		10-85	Revision	Sheet No.	Index No.	
Checked By			00	4 of 4	17882	

TYPICAL BRIDGE MOUNTS

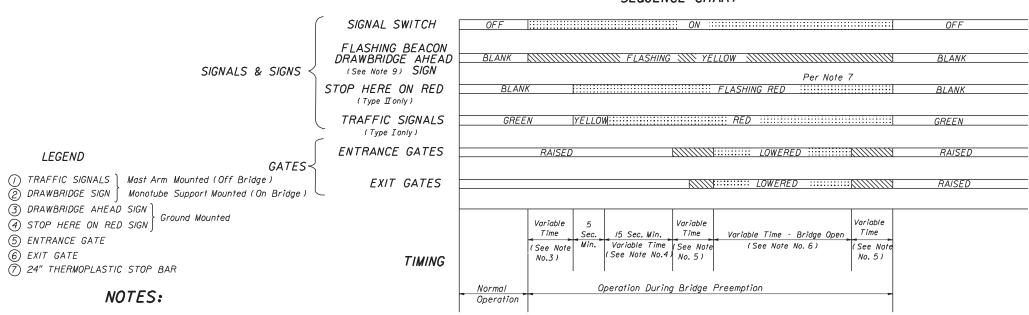


* Field conditions may require adjustment of this standard distance.

TO BE USED WHERE BRIDGE OPERATORS ARE FULL TIME OR A DAILY BASIS

TO BE USED WHERE TYPE I IS NOT APPLICABLE (USUALLY WHEN THE BRIDGE OPERATOR IS "ON CALL")

SEQUENCE CHART

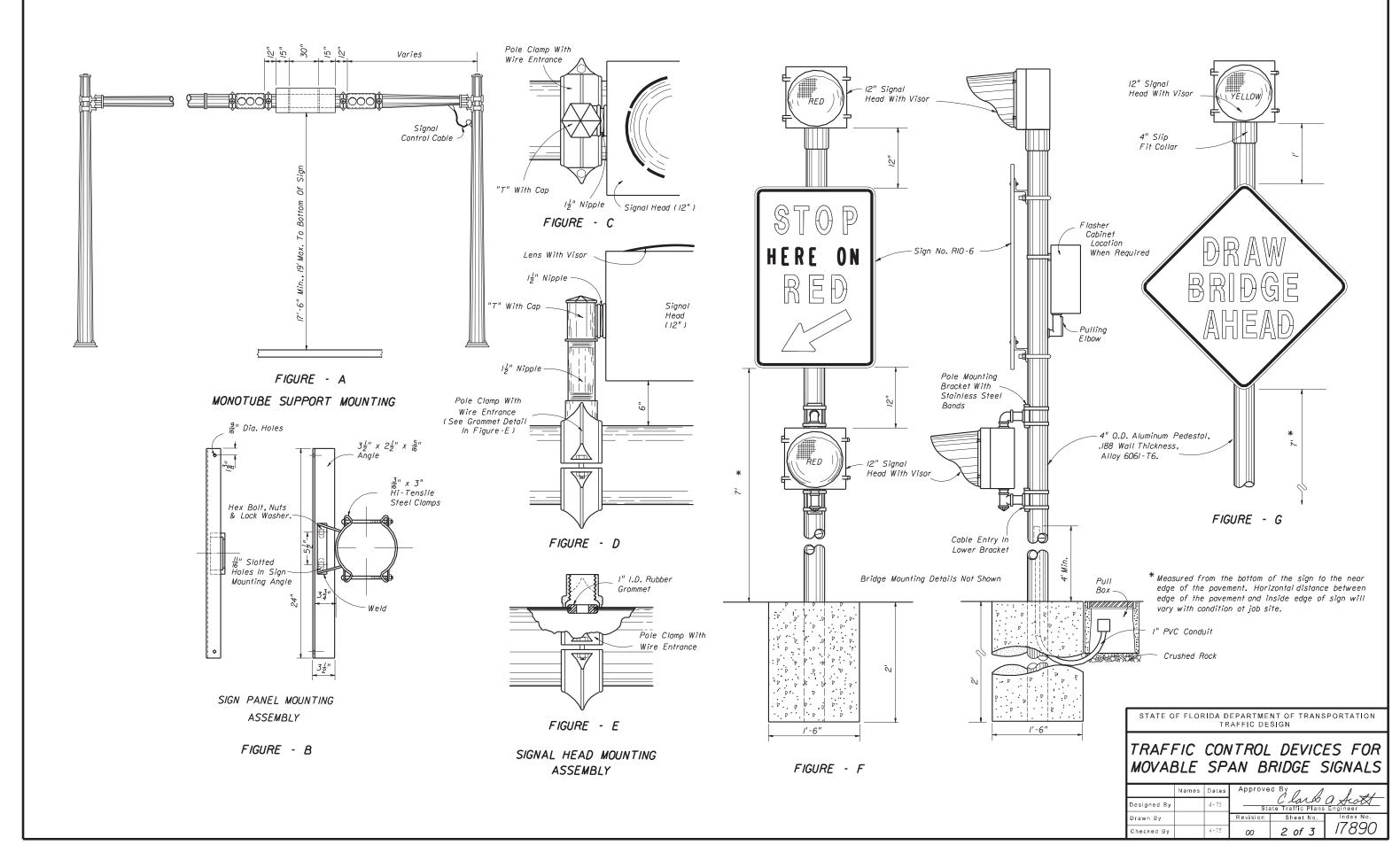


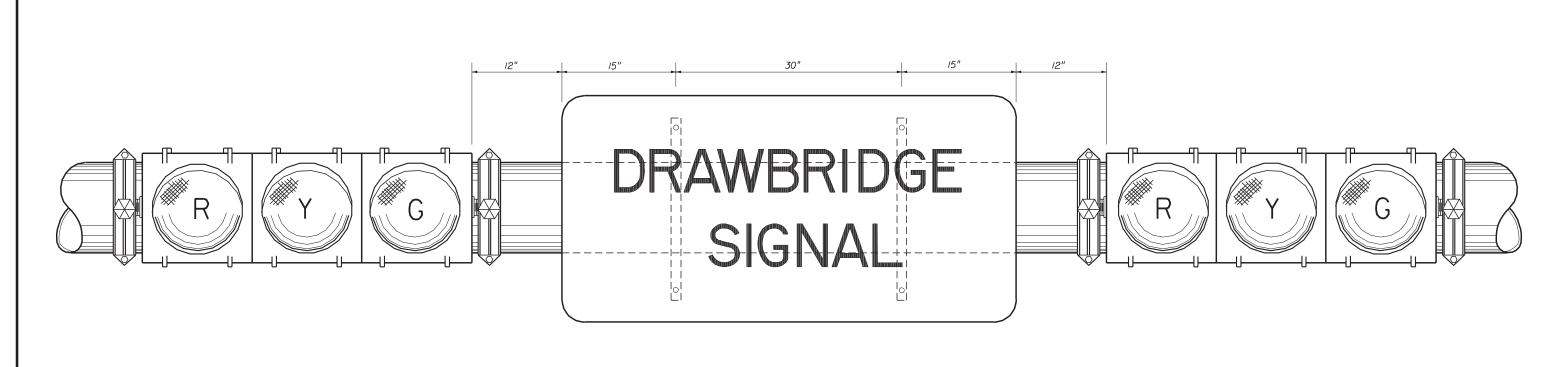
- I. A bypass switch shall be installed to override each timing interval in case of a malfunction.
- 2. "STOP HERE ON RED" is omitted in Type I operation and "TRAFFIC SIGNALS" are omitted in Type II operation.
- 3. The time between beginning of flashing yellow on "Drawbridge Ahead" sign and the clearance of traffic signal to red, or beginning of flashing red should not be less than the travel time of a passenger car, from the sign location to the stop line, traveling at the 85 percentile approach speed.
- 4. Beginning of operation of drawbridge gates shall not be less than 15 seconds after steady red or 20 seconds after flashing red (Actual time may be determined by the bridge tender.)
- 5. Time of gate lowering and raising is dependent upon gate type.
- 6. Time of bridge opening is determined by the bridge tender.
- 7. Each gate shall be operated by a separate switch.
- 8. On each approach (Type II), all four red signals shall be on the same two circuit flashers, with the two top signals on one circuit, and the two bottom signals on the alternately flashing circuit.
- 9. A Drawbridge Ahead sign is required for both types of signal operation, However a flashing beacon shall be added to the sign when physical conditions prevent a driver traveling at the 85% approach speed from having continuous view of at least one signal indication for approximately IO seconds.
- 10. Requirements on gate installation are contained in Section 4E-14 through 4E-17 of the Manual on Uniform Traffic Control Devices.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION
TRAFFIC DESIGN

TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS

	Names	Dates	Approve	1 1 11	- 1-41	
Designed By		4-75	State Traffic Plans Engineer			
Drawn By		4-75	Revision	Sheet No.	Index No.	
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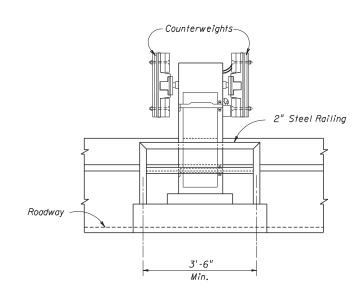


5' x 2'-6" 2" Border-4" Radius 6" Series "D" Letters

BLACK OPAQUE LEGEND AND BORDER ON REFLECTORIZED YELLOW BACKGROUND

TO BE USED WITH TYPE I OPERATION, AS SHOWN
ON PREVIOUS SHEET

MONOTUBE SUPPORT MOUNTING



Edge Of Travelway

Roadway

 \bigcirc

2' Min.

Down Position

5'-0" Min.

GATE & ARM DETAIL

Class I Or II (Length Shall Be Shown On Plan Sheets) I'-6" Center See Note 2 R/R & Drawbridge Arms 18' to 20' Center Line Mast R/R & Drawbridge Arms 32' And Over

Note :

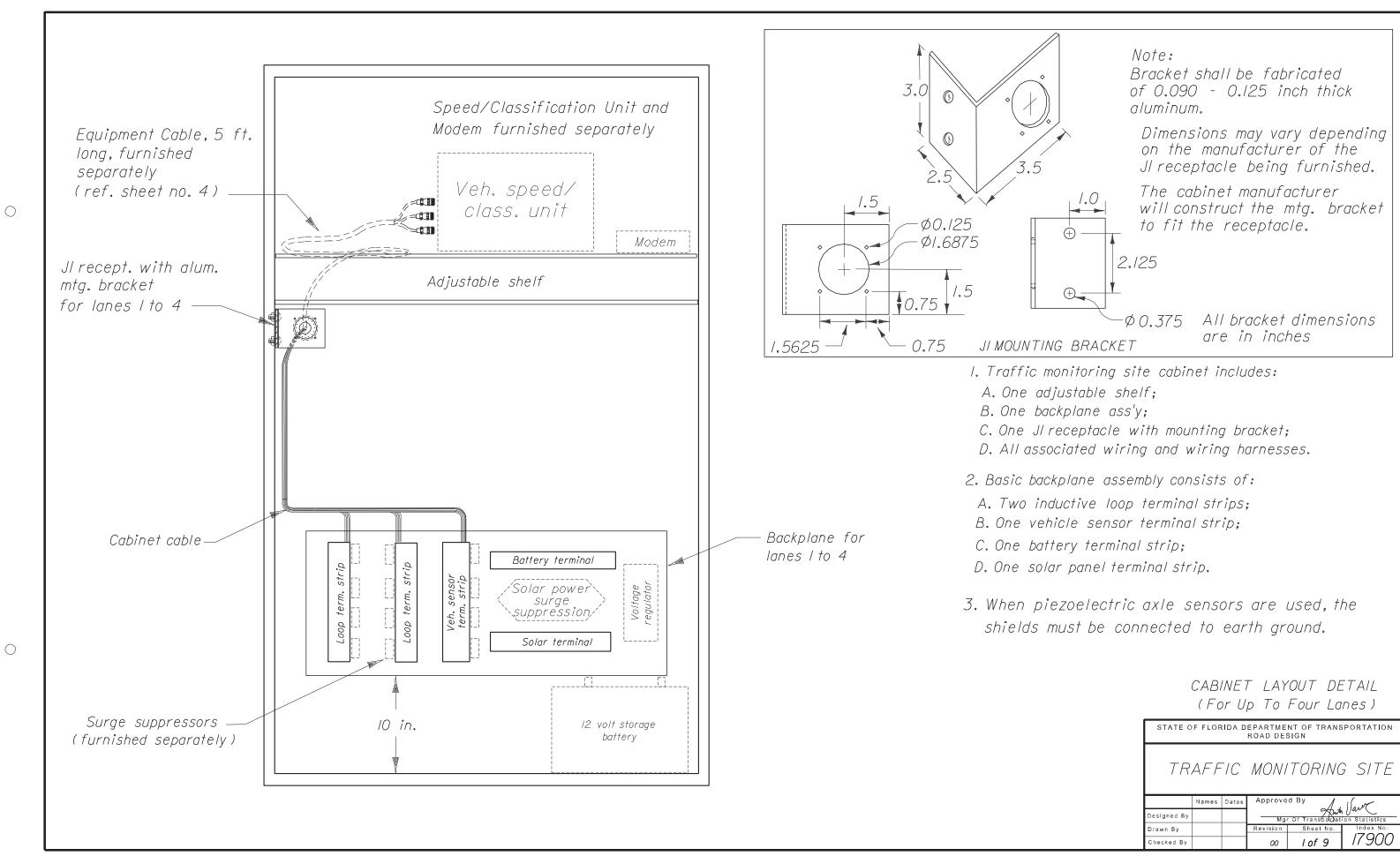
- I. 12 volt flashing red lights shall be mounted on gate arm and shall operate in the flashing mode only when gate arm is in the lower position or in the process of being lowered. The number of lights shall vary accordingly to length of the gate arm.
- 2. 16" alternate diagonal fully reflectorized red and white stripes.

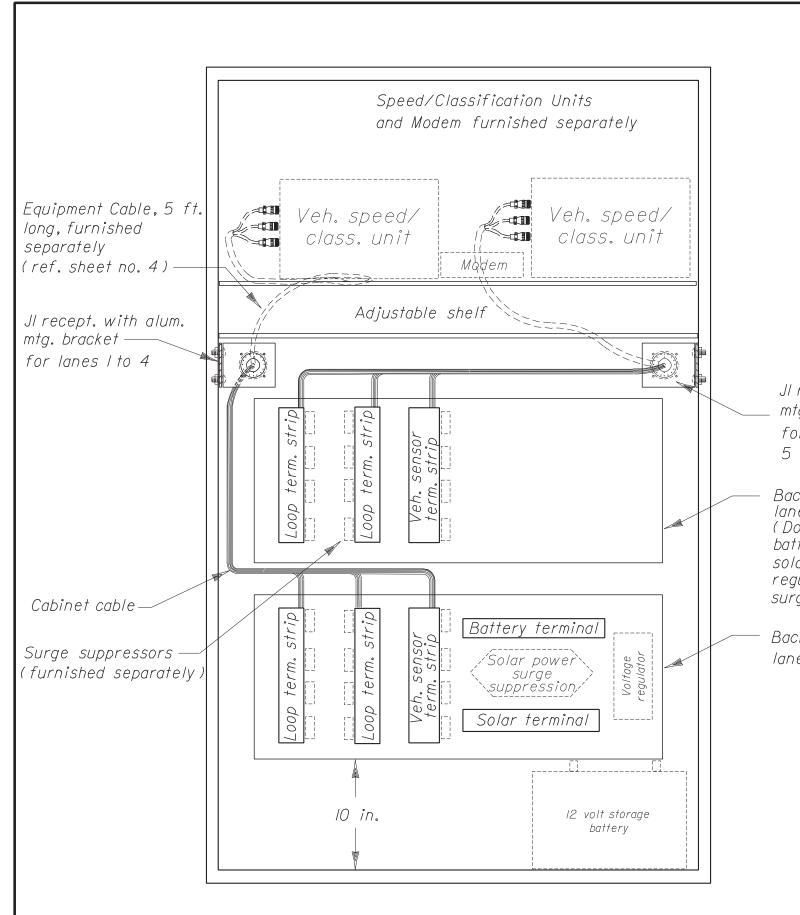
TYPICAL LAMP PLACEMENT

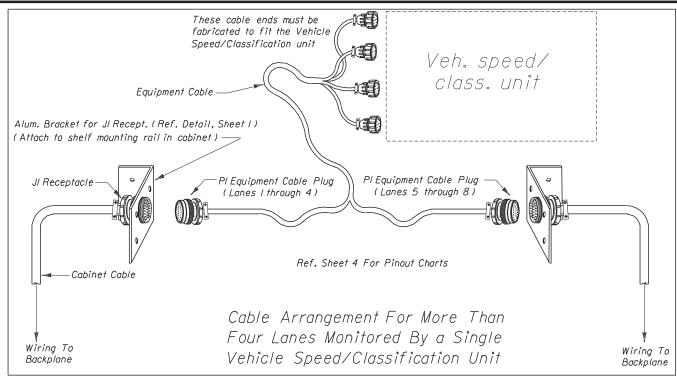
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION TRAFFIC DESIGN

TRAFFIC CONTROL DEVICES FOR MOVABLE SPAN BRIDGE SIGNALS

	Names	Dates	Approve		- 1 -1	
Designed By			Clark a Acott State Traffic Plans Engineer			
Drawn By			Revision	Sheet No.	Index No.	
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JI recept. with alum. mtg. bracket for lanes 5 to 8

Backplane for lanes 5 to 8 (Does not require battery terminal, solar terminal, voltage regulator, or solar power surge suppressor.)

Backplane for lanes I to 4

- I. Traffic monitoring site cabinet includes:
- A. One adjustable shelf;
- B. Two backplane assemblies (equipped as shown);
- C. Two JI receptacles with mtg. brackets;
- D. All associated wiring and wiring harnesses.
- 2. Basic backplane assembly consists of:
- A. Two inductive loop terminal strips;
- B. One vehicle sensor terminal strip;
- C. One battery terminal strip;
- D. One solar panel terminal strip.
- 3. When piezoelectric axle sensors are used, the shields must be connected to earth ground.

CABINET LAYOUT DETAIL

(For More Than Four Lanes

And Up to Eight Lanes)

TRAFFIC MONITORING SITE

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

Designed By

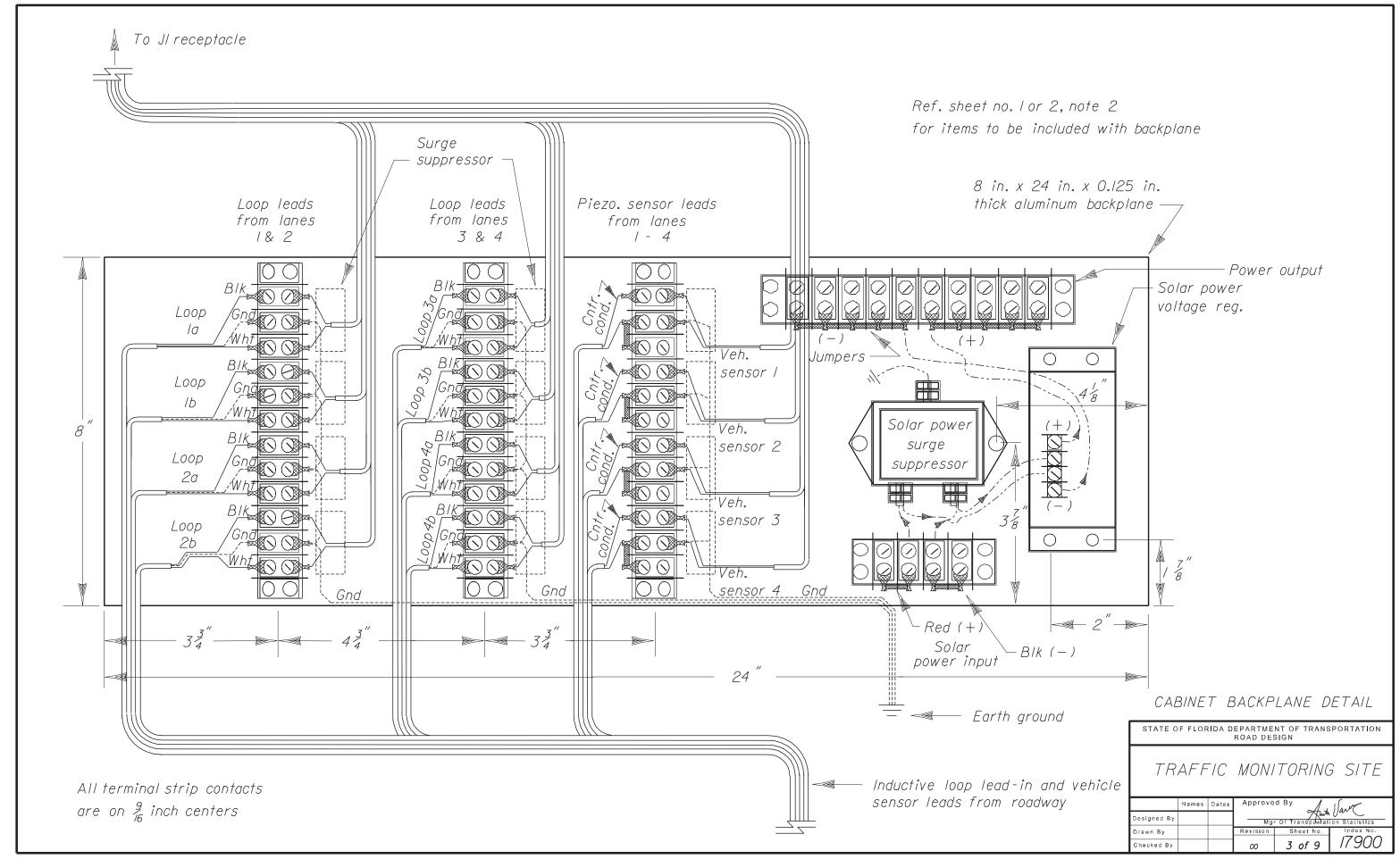
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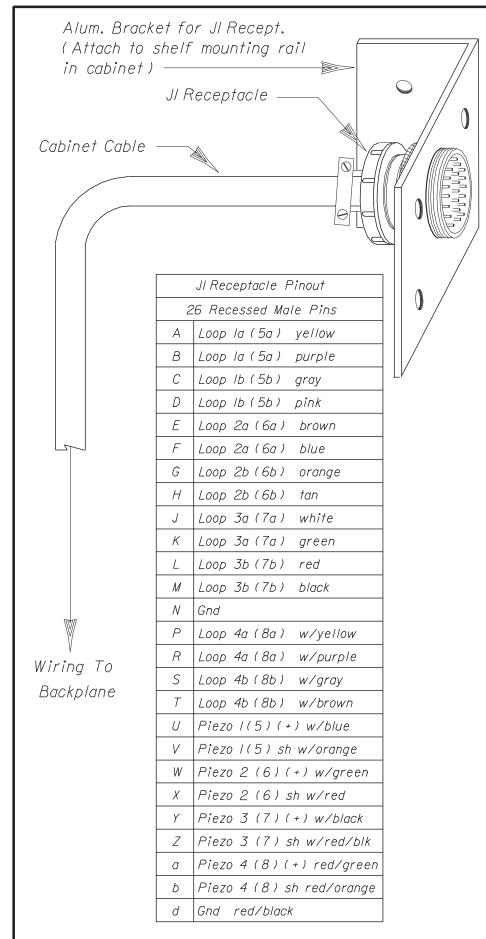
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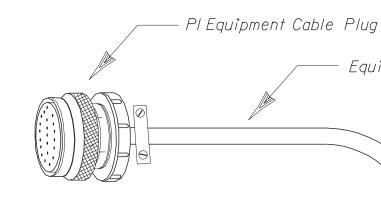
Names Dates Approved By

Mgr Of Transposation Statistics

Revision Sheet No. Index No. 17900

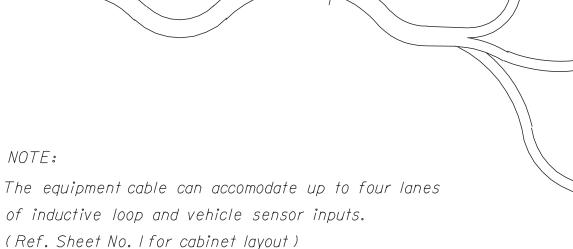






	guipment Cable Plug	-			
26	Female Pin Slots				
Α					
В	Loop Ia (5a)				
С	Loop 16 (5b)	to 4			
D	Loop Ib (5b)	Connects to ectronics un			
Ε	Loop 2a (6a)	nne			
F	Loop 2a (6a)	Co			
G	Loop 2b (6b)	ه ا			
Н	Loop 2b (6b)				
Ν	Gnd				
J	Loop 3a (7a)				
K					
L Loop 3b (7b)					
M Loop 3b (7b)					
Р	Loop 4a (8a)	Connects to ectronics un			
R	Con/ e/ectr				
S	6/6				
T					
d	Gnd				
U	Piezo /(5) (+)				
V	Piezo Ish				
W	to .:				
Χ	Connects to electronics un				
Υ	tron				
Z Piezo 3 sh					
а	Piezo 4 (8) (+)	\rceil			
Ь	Piezo 4 sh				

These cable ends must be fabricated to fit the Vehicle Speed/Classification unit



For more than four lanes and up to eight lanes of inputs, the following options are available:

Equipment Cable

- I. A second Vehicle Speed/Class. Unit and separate equipment cable connecting to a second JI receptacle; or
- 2. A single Vehicle Speed/Class. Unit capable of up to eight lanes of inputs and a single equipment cable with split ends to fit two JI receptacles. (Ref. Sheet 2 detail)

Numbers in parenthesis in the pinout chart identify lane numbers when a second backplane for lanes 5 through 8 is required.

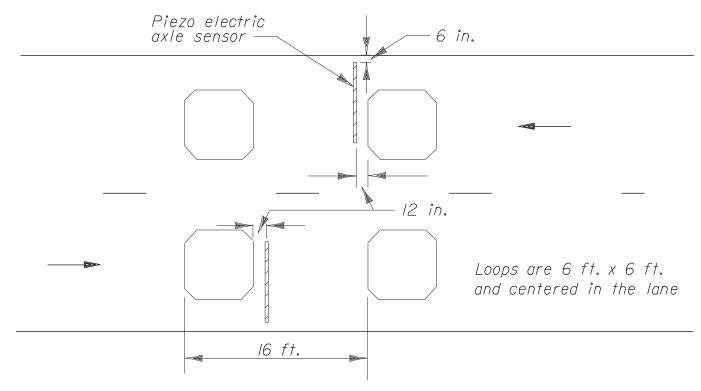
EQUIPMENT CABLE DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC MONITORING SITE

	Names	Dates	Approve	d By	i Vare
Designed By			Mgr Of Transportation Statistics		
Drawn By			Revision	Sheet No.	Index No.
Checked By			00	4 of 9	17900

SPEED/CLASSIFICATION LOOP ASSEMBLY WITH AXLE SENSORS PLACEMENT DETAIL



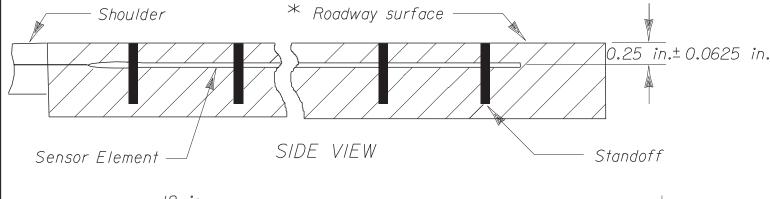
Note:

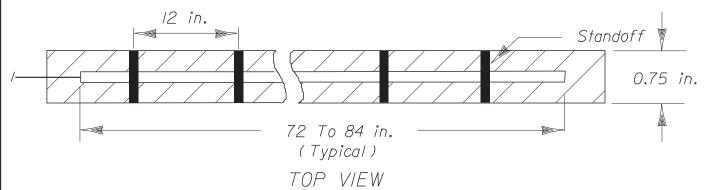
Loop slots shall be 0.25 inches wide (approx.) by 1.5 inches to 2 inches deep. Three turns of #12 AWG, type XHHW stranded copper wire shall be placed in the slot. Backer rod shall be used to hold the loop wire in the bottom of the slot.

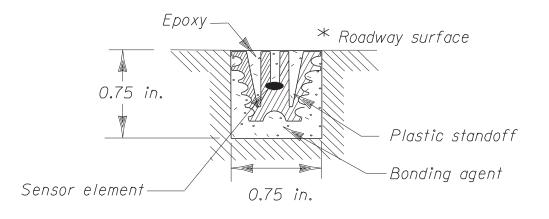
Loop leads shall be twisted at the rate of 10 to 12 twists per foot. The twisted pair shall extend to the pull box with three feet of spare length coiled in the pull box.

All leads (inductive loop & vehicle sensor) shall be identified according to the lane numbering convention shown on sheet 8 and 9.

TYPICAL UNENCAPSULATED CLASS II VEHICLE SENSOR







END VIEW

* Some installations may require axle sensors to be placed in the structural course, prior to placement of the friction course.

Note:

These are typical dimensions. actual dimensions, element cross-sections and standoffs may vary depending on manufacturer and model.

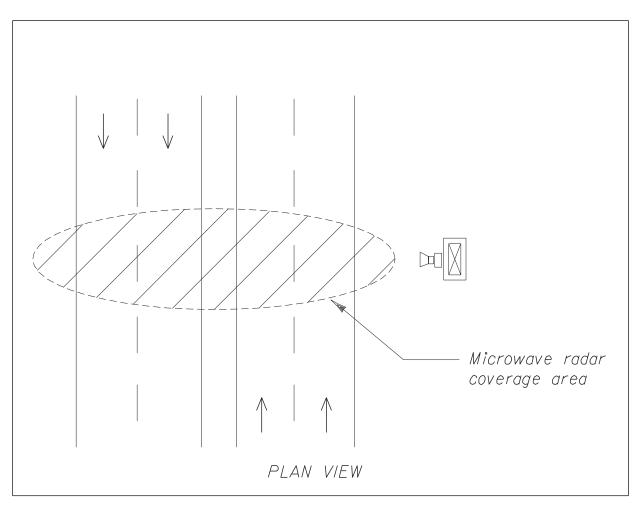
LOOP AND PIEZOELECTRIC VEHICLE SENSOR DETAIL

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC MONITORING SITE

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Solar Power Pole (Ref. ROADWAY & TRAFFIC DESIGN STANDARDS 17900) -

Microwave Radar Vehicle Sensor
(Type II Vehicle Sensor - Typical Mounting)
Mounting height must be adjusted to optimize
the unit's coverage area.

Drill a $\frac{1}{2}$ to $\frac{5}{8}$ inch dia.

hole in the pole for sensor lead access. Pull leads through pole cavity and into the cabinet.

16' - 23'

ROADWAY

min.

The unit must be capable of detecting up to eight lanes of traffic (in either or both directions) when mounted perpindicular to the roadway.

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Coverage area of the unit is affected by the roadway geometry: distance from the travel lanes, median type and width, barrier walls, etc.

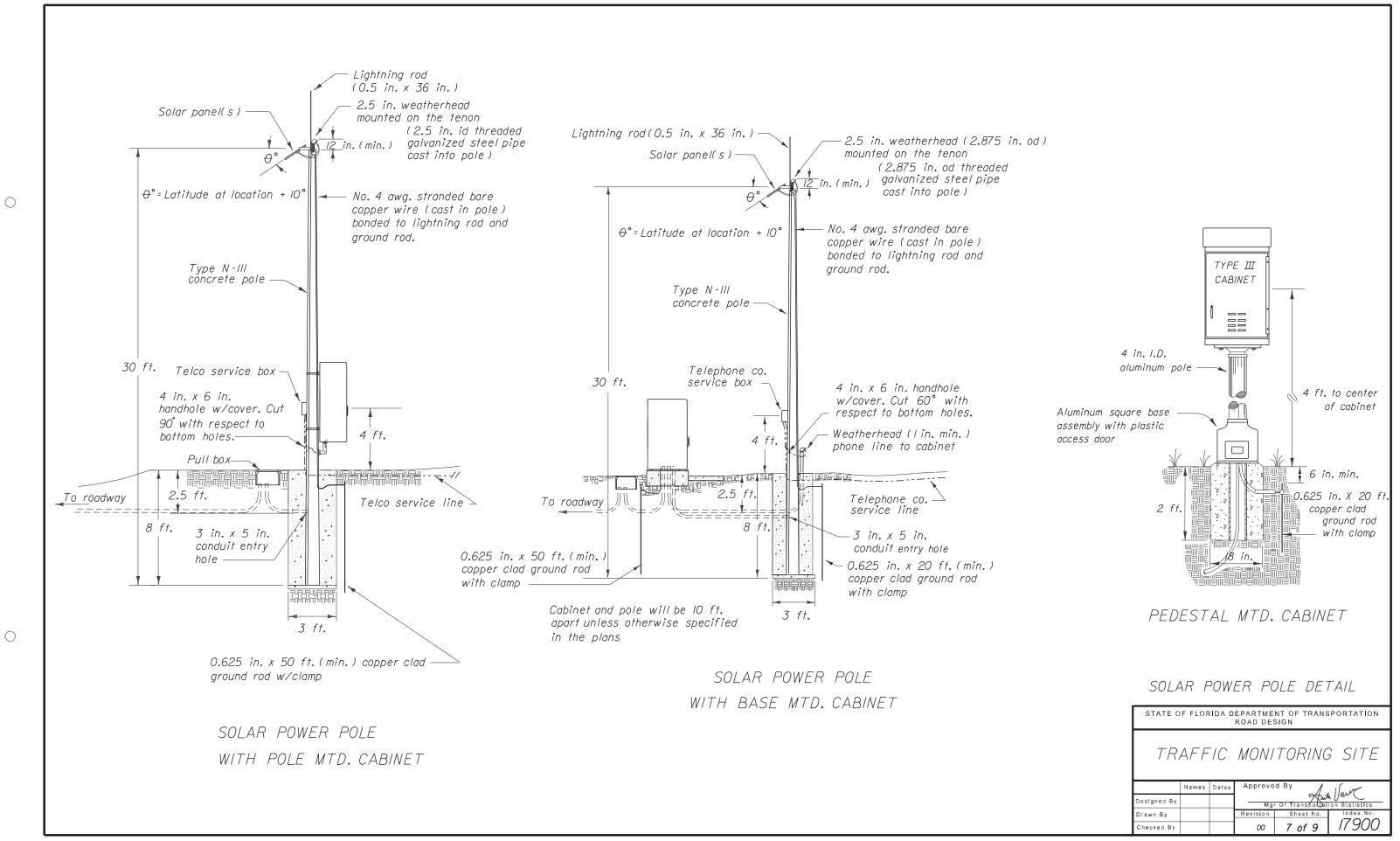
Mounting height of the unit and offset from the roadway must be determined on a site-by-site basis. 5' is the minimum operable offset and not a standard.

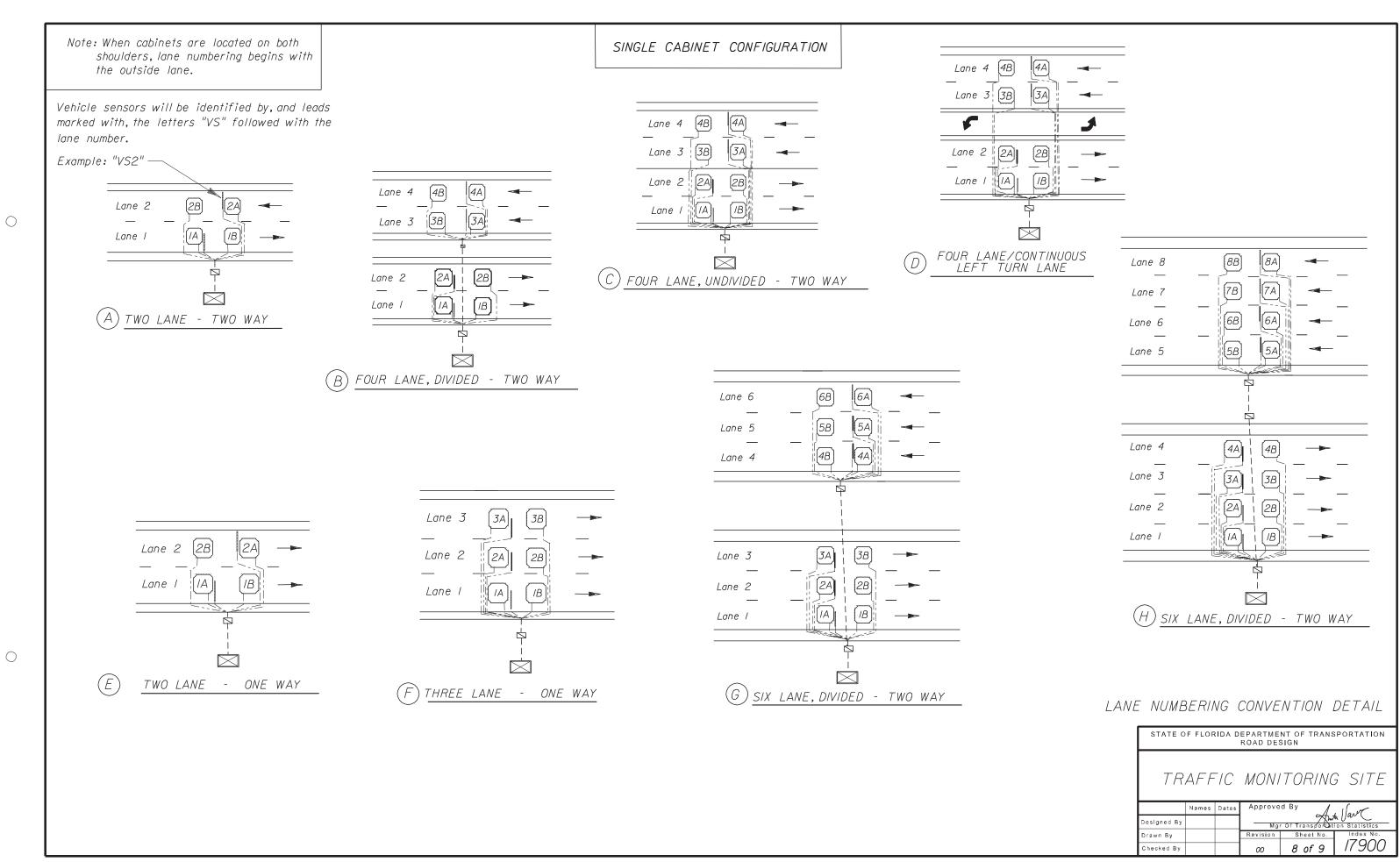
TYPE II VEHICLE SENSOR
MICROWAVE RADAR

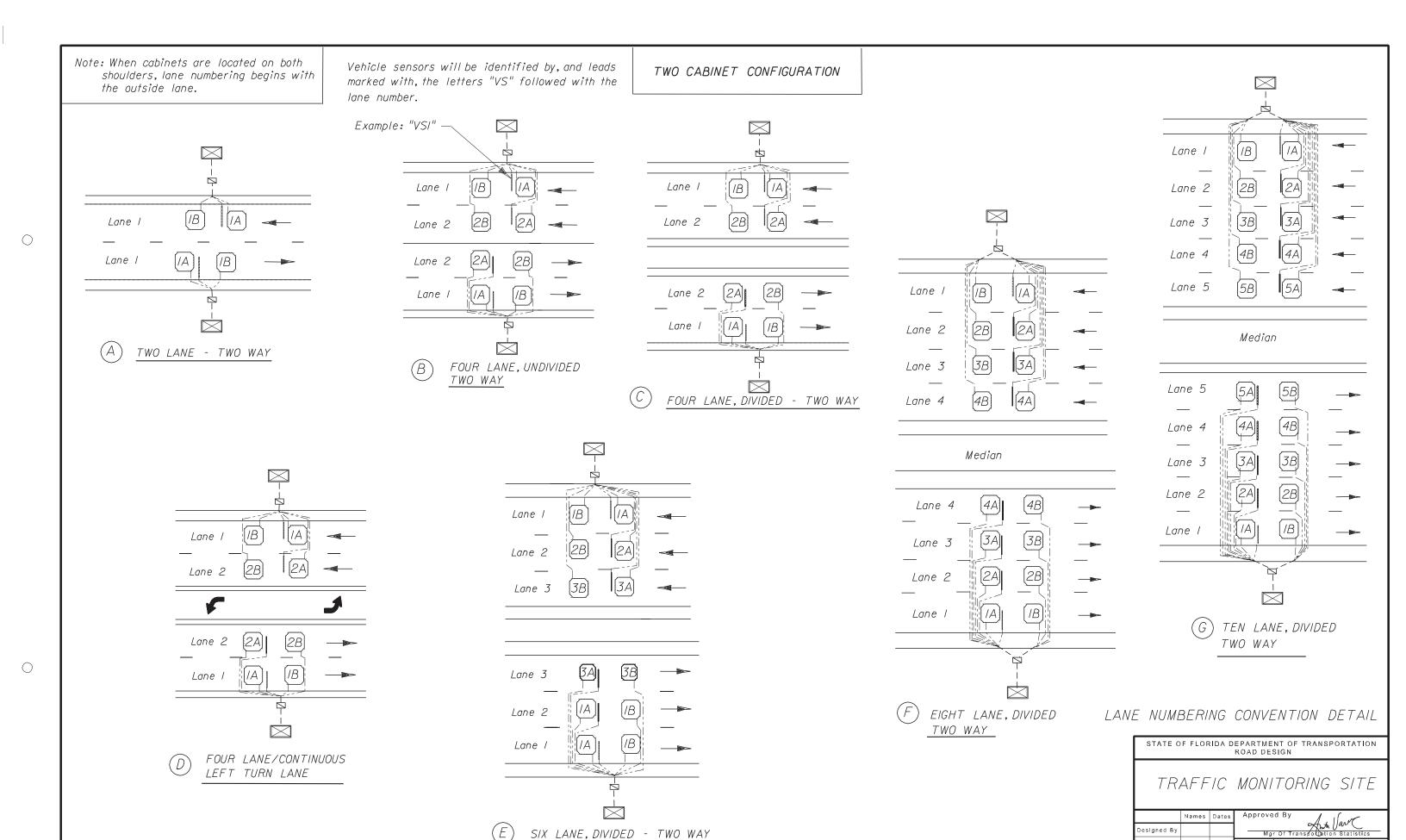
STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION ROAD DESIGN

TRAFFIC MONITORING SITE

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