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EXECUTIVE SUMMARY

The primary objective of this research was to test in the field a new method for installing break-away slip-base connections. The new method was developed in the laboratory in a previous phase of the research, and it is based on the utilization of Belleville spring washers. Stacks of spring washers, which come pre-wrapped, are installed on the bolts just as any ordinary washers. The bolts are torqued, and a block of filler gages measures the deflection of the stack of spring washers which is proportional to the tension in the bolt.

Seven signs were selected along Central and South Florida highways. At each sign, one of the posts was equipped with bolts with spring washers, while the other post remained connected with bolts with conventional flat washers. At each sign one of the bolt with spring washers and one of the bolts with flat washers were instrumented with strain gages. The variations of tension in these bolts were monitored over a period of one year by measuring the tension of the different bolts once a week.

The measurements showed that, at the time of installation, the new method was significantly more effective in ensuring the proper tension in the bolts. Similarly, over the long term, the bolts installed with spring washers maintained more effectively the tension in the bolt, without any loosening of the bolt. On the contrary, several bolts installed with flat washers exhibited a downward trend in the bolt tension.

The effectiveness of the break-away connection with spring washers was also demonstrated during a car accident. A car hit a sign post equipped with spring washers, near the Sebastian Inlet. The break-away functioned perfectly and the post separated from the base.

The opinions, findings and conclusions expressed in this publication are those of the authors and not necessarily those of the U.S. Department of Transportation.

This report was prepared in cooperation with the State of Florida Department of Transportation and the U.S. Department of Transportation.

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INTRODUCTION

For the efficient control of traffic on highways it is necessary to provide large directional signs. These signs require massive fixed supports which, when located near the roadway, constitute a hazard to the occupants of an errant vehicle. To reduce the hazard, if the signs cannot be located behind a guardrail, the sign supports must be provided with a break-away device.

According to a detailed, mid-60's study of break-away signs performed by Edwards, Hirsch, and Olson at the Texas Transportation Institute, the *bolt tension is the single most critical factor for the safe operation of the break-away sign support*. In fact, the bolts in the slip-base connection must be tightened to a specific tension, in order to satisfy two conflicting requirements: 1) to ensure that the connection will successfully withstand the service loads due mainly to wind, traffic, and temperature effects; and 2) to ensure that the connection will break upon impact from an errant vehicle, thus ensuring the safety of the vehicle's passengers. Consequently, there is a need to accurately and directly measure the tension in the bolts, when the break-away system is installed.

Equally important, is a need to ensure that the tension in the bolts will remain within an acceptable range throughout the service life of the system. Even if the bolts were correctly installed with the proper tension, the bolts could still loosen over time. This arises from the fact that highway signs are subjected to a variety of dynamic loads including the vibrations induced by traffic, and the effect of wind induced vortex shedding. These vibrations are transmitted to the supporting structure and can result in a loosening of the base bolts. Temperature changes due to daily and seasonal changes can also cause non-uniform expansion and contraction of the break-away assembly introducing residual tension in the bolts. In the case of excessive bolt loosening, the base sign connection could fail during a wind event, result in a hazard and disruptions to highway traffic, and result in injury for the occupants of a vehicle, and costly liability for the responsible jurisdiction. In addition, the losses or damage to traffic signs are costly.

To solve the above problem, a two phase research program on break-away signs was carried on at Florida Tech, with funding from the Florida DOT. Phase I of the program addressed the first need listed above, by developing a new installation procedure for the bolts of the slip-base

connections that will replace the calibrated wrench method currently being used. The new installation procedure determines the tension in the bolt by measuring the deflection of a stack of spring washers when installing the bolt.

Phase I of the research program also included laboratory tests on the effect of vibration and temperature variations on the loosening of the bolts in the break-away. It was shown in the laboratory that the spring washers substantially reduced the loosening of the bolts.

Phase II of the research program intended to verify, in the field, the results developed in the laboratory during Phase I. The objectives of Phase II were the following:

- To verify the long-term behavior of the Belleville spring washers, specifically their capacity to maintain the bolt tension within the acceptable range.
- To compare the behavior of the sign post break-away base installed with spring washers with the behavior of the sign post break-away base installed with regular flat washers.
- To determine the longevity of the galvanized and stainless steel spring washers in the open field Florida environment.

FIELD TESTS SET-UP

Instrumentation of Bolts

- Two bolts were used: 3/4" –A235 bolts with two different lengths of 3" and 3 1/2"; and 7/8" bolts also with lengths of 3" and 3 1/2". The 3" bolts were used with standard washer set-ups while the 3 1/2" bolts were used with the proposed Belleville washer set-ups.
- The bolts were instrumented with two strain gages each. The strain gages were mounted 180
 degrees apart in the bolt shank in machined grooves in order to protect the strain gages from
 contact with the plates and the washers of the break-away connection. The strain gages were
 instrumented in this fashion so that the average of the two strain readings would cancel any
 bending effects, which may occur in the bolt shaft.
- Two holes were drilled symmetrically in the bolt head above the two grooves to let electrical leads pass. Figure 1 shows an example of an instrumented bolt.
- The EA-06-240LZ-120 type of strain gages manufactured by Micro-Measurements Group Inc. was used. The gage resistance in ohms at 24 °C is 120.0 ± 0.3% and a gage factor at 24°C is 2.055± 0.5%.
- To take strain measurements, the gages were connected using a quarter bridge pattern to a P-3500 strain gage indicator which has an accuracy of ±0.1% for a gage factor greater than 1.
 The strain gage indicator was also manufactured by Micro-Measurements Group Inc.

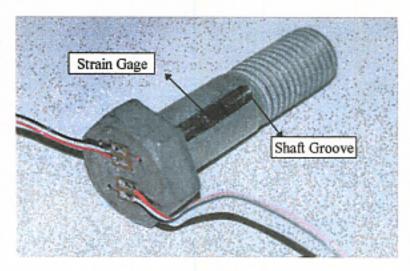


Figure 1 - 3/4" Diameter Instrumented Bolt

Location and Details of Selected Signs

The first step taken was to select suitable sites throughout Central Florida considering different conditions of wind, traffic and temperature. Seven appropriate sites were identified along I-95, I-4, I-75 and A1A, which are typical of different climate and traffic conditions in central and south Florida. At each site, representative break-away signs were selected for monitoring in coordination with the Florida DOT. Figure 2 and Table 1 show the location and details of the selected sites.

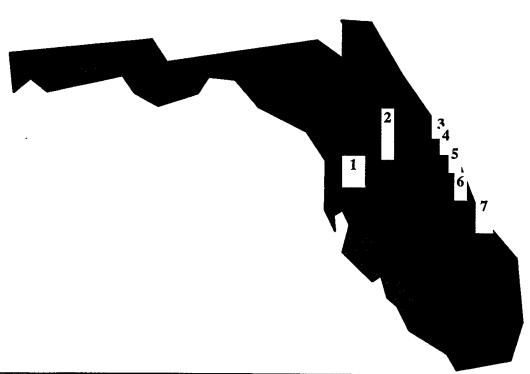
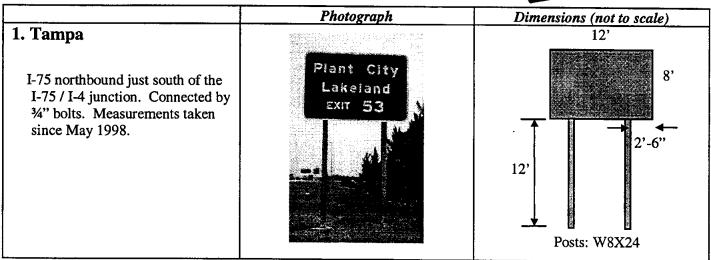
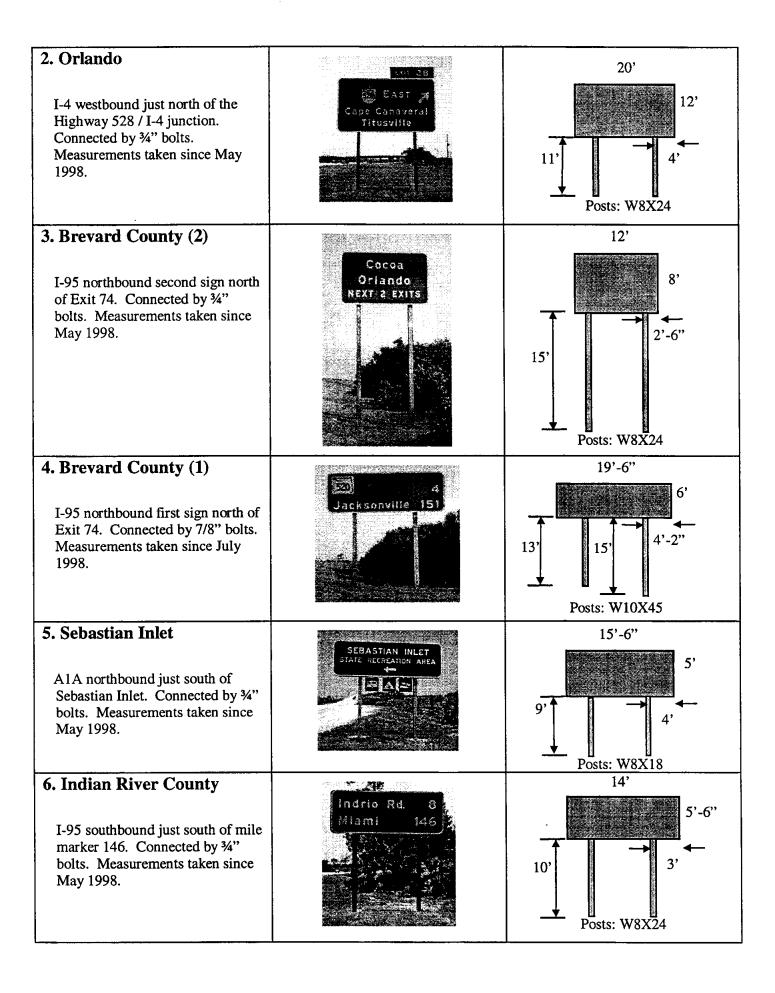


Figure 2 Location of Selected Signs





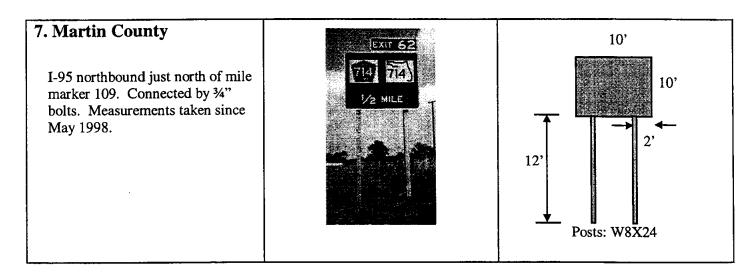


Table 1: Location and Details of Selected Signs

Installation of Bolts

After the break-away signs were selected, the instrumented bolts were installed. Each sign had two posts, each with four 3/4" bolts except for Brevard (1) which has 7/8" bolts. The bolts on one post were installed using the procedure defined in Phase I [1] with the proposed Belleville spring washer set-up, using the 3.5 in long bolts. The Belleville spring washer set-up used included four washers (# AI-1575107), two in series and two in parallel. This set-up requires a deflection of 0.026" to reach FDOT recommended bolt tension. Deflections were measured using filler gages. All material (washers, bolts, and nuts) was provided by FDOT with the exception of the spring washers, which was purchased by the research team. The actual installation was done by the research team with assistance from FDOT crew.

The bolts on the other post were installed following the conventional torquing method and washer set-up, using the 3 in. bolts. The bolts installed using this method were torqued to either 35 ft-lb or 43 ft-lb according to individual FDOT specifications. It can be seen that not all sites use the same torque values. Table 2 shows the specific torques for each of the signs.

Table 2: Recommended Torque Values

Sign	Recommended Torque (ft-lb)			
Tampa	43			
Orlando	35			
Brevard County	43			
Sebastian Inlet	43			
Indian River County	43			
Martin County	43			

Each post was equipped with one instrumented bolt so a comparison could be made between the conventional set-up and the proposed set-up. Figures 3 and 4 compare the traditional set-up using conventional flat washer and the proposed set-up using spring washers. The requirement was for the washers to maintain the tension within 2400 lb. to 3600 lb.

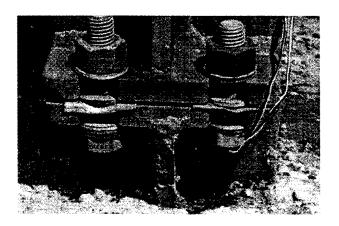


Figure 3 - Traditional Bolt Set-Up with Flat Washers

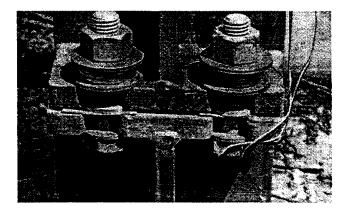


Figure 4 - Proposed Set-Up with Spring Washers

Strain Reading Procedure

- 1. The signs were checked once a week on average. The Indian River, Martin and Sebastian signs are checked one day while the two Brevard signs along with the Orlando and Tampa signs are checked another week day. Sometimes, the checking could not be carried out because of the weather or because the bolts are not accessible due to flooding.
- 2. The procedure was as follows:
- Measured the temperature using a BAT-10 Thermometer made by Physitemp, Inc.
- Checked for rusting on the spring washers.

- Checked if the strain gages were working (each gage should register a resistance of 120 Ω).
- Checked the offset readings and adjust the strain indicator accordingly. The offset readings were determined by setting the strain indicator readings to zero when the bolt is in zero tension.
- Measured the strains on the left and right strain gages.
- Replaced the wires in a sealed plastic bag and the bag is taped onto the signpost.
- 3. The measurements were placed in a spreadsheet. The spreadsheet calculated the tension and the eccentricity on each bolt.
- 4. If some of the instrumented bolts were damaged, the bolt was removed and replaced with an appropriate regular bolt. The instrumented bolt was brought back, repaired, re-tested in the lab, and re-installed in the field the following week.

Calculation of Tension and Eccentricity

The monitoring of the bolt tensions began as soon as the bolts were installed. The results show how the tensions in the bolts changed since they were installed. Table 3 is an example of spreadsheet results.

Operator	Date	Time	Temp	Strain Reading (με)			Bolt tension		Eccent.	
			ိင	ိင	°C Left	Right	Average	lb.	kN	(in)
			Balance Offset >>	392	59		·····			
S.ON.	7/16/98	9:00	30	284	55	170	2956	13.1	0.074	
S.ON.	7/23/98	9:45	32	338	60	199	3470	15.4	0.076	
N	8/11/98	1:00	36	237	57	147	2563	11.4	0.067	
N	8/26/98	10:30	35	265	92	179	3113	13.8	0.053	

Table 3: Brevard (1) Sign, Bolt with Spring Washer (Sample)

The averages of the strain measurements were used to calculate the tensions on each bolt. The eccentricity measures the distance between the bolt tension force and the centroid of the bolt.

The equations used are:

Tension =
$$T = E \varepsilon_{ove} A$$

Where:

E = Young's modulus measured in the lab. On average, it is equal to 2.9×10^7 psi

A = area of the bolt
$$= \pi \left(\frac{0.75}{2}\right)^{2} \qquad for the \frac{3}{4}$$

$$= \pi \left(\frac{0.875}{2}\right)^{2} \qquad for the \frac{7}{8}$$
bolt

$$Eccentricity = \frac{d}{8} \frac{\varepsilon_L - \varepsilon_R}{\varepsilon_L + \varepsilon_R}$$

Where:

d = diameter of the bolt

Figure 5 illustrates the relationship between tension, strain and eccentricity.

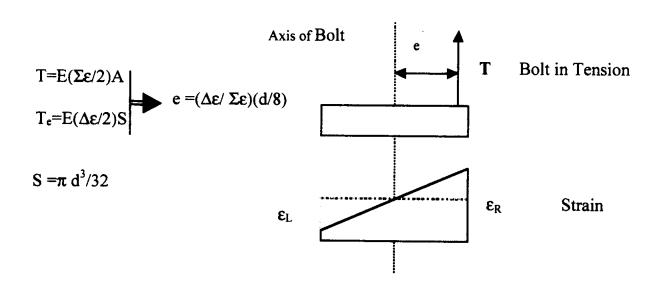


Figure 5 - Bolt Tension Eccentricity

TENSILE TESTS

Prior to installation of the instrumented bolts in the field, tensile tests were performed in the lab, to ensure that the strain gages had been properly installed. The tests were performed following the technique described in the Phase 1 report [1]. In each case, the goal was to verify that the measured modulus of elasticity was close to the theoretical value of 29,000 ksi. Every time a bolt was brought back to the lab for re-instrumenting, a new tensile test was performed prior to re-installing.

Below are the results of the tests for each instrumented bolt. The stress-strain curves are plotted for each bolt. In each figure, the readings from each strain gage are plotted with triangular dots for the left gage and square dots for the right gage. The average values are represented with a solid line. The slope of this line or measured modulus of elasticity is indicated in each plot.

Tampa

Figure 6 shows the result of the tensile test of May 18, 1998 for the short bolt (to be installed with flat washers).

Figure 7 shows the results of the tensile test of May 18, 1998 and the tensile test for the long bolt (to be installed with spring washers) of December 8, 1998.

Orlando

Figures 8 and 9 show the results of the tensile tests of May 16, 1998 and December 1, 1998 for the short bolt.

Brevard (2)

Figures 10, 11, and 12 show the results of the tensile tests of May 16, 1998, November 10, 1998 and January 22, 1999 for the short bolt.

Brevard (1)

Figures 13 and 14 show the results of the tensile test of July 14, 1998 for both bolts and Figure 15 shows the tensile test for the long bolt of November 19, 1998.

Sebastian Inlet

Figures 16 shows the results of the tensile test of December 8, 1998 for the long bolt and Figure 17 shows the results of the tensile test of December 3, 1998 for the short bolt.

Indian River

Figures 18 and 19 show the results of the tensile tests of May 15, 1998 and February 3, 1999 for the long bolt.

Figures 20, 21, and 22 show the results of the tensile test of May 15, 1998, December 8, 1998 and February 2, 1999 for the short bolt.

Martin County

Figure 23 shows the results of the tensile test of December 3, 1998 for the long bolt and Figure 24 shows the results of the tensile test of December 3, 1998 for the short bolt.

Tampa Sign, Bolt w/ Flat Washers E = 29.683 x 10⁶ psi Lt Gade Stress (psi) Strain (µE)

Figure 6 Stress vs Strain, Tampa Sign, Bolt with Flat Washers, May 18, 1998

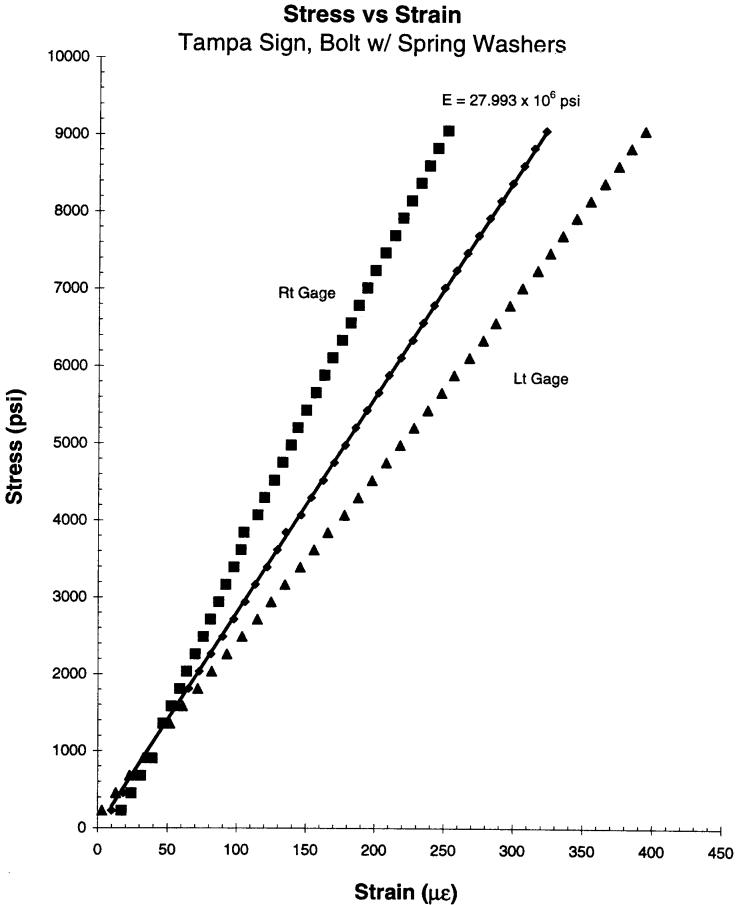


Figure 7 Stress vs Strain, Tampa Sign, Bolt with Spring Washers, December 8, 1998

Stress vs Strain Orlando Sign, Bolt w/ Flat Washers E = 28.42 x 10⁶ psi Rt Gage Stress (psi) Strain (με)

Figure 8 Stress vs Strain, Orlando Sign, Bolt with Flat Washers, May 16, 1998

Orlando Sign, Bolt w/ Flat Washers

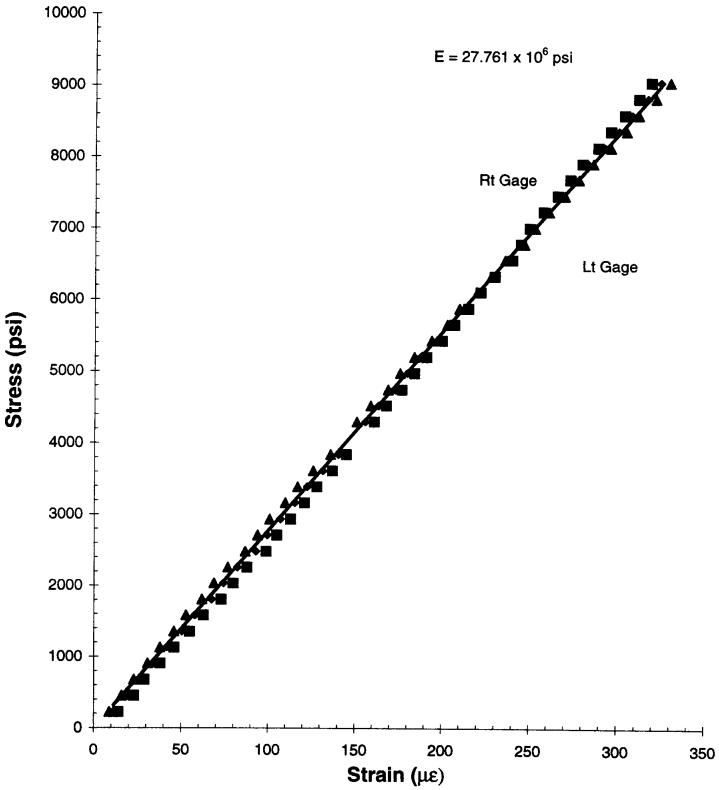


Figure 9 Stress vs Strain, Orlando Sign, Bolt with Flat Washers, December 1, 1998

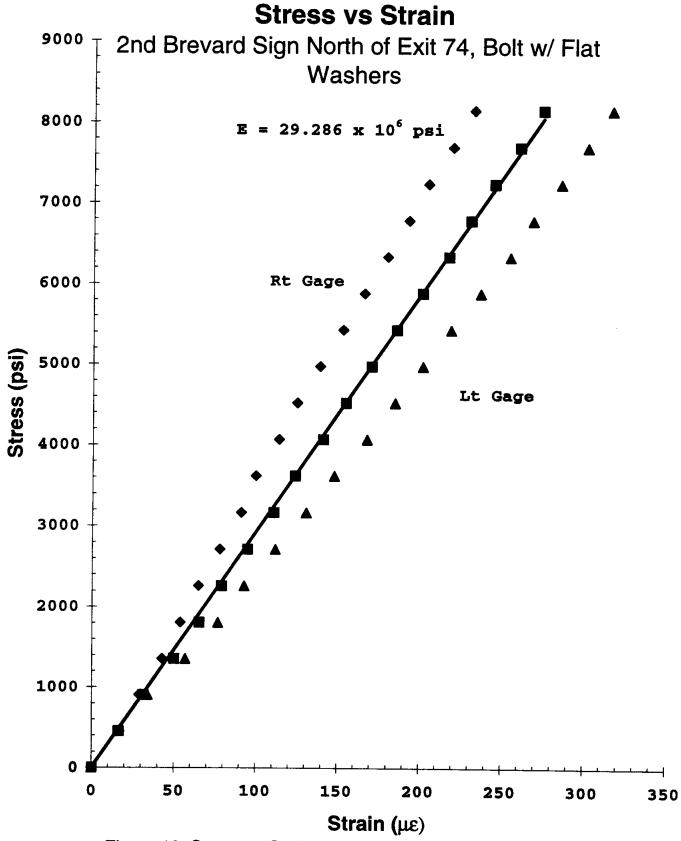


Figure 10 Stress vs Strain, Brevard (2) Sign, Bolt with Flat Washers, May 16, 1998

Stress vs Strain
2nd Brevard Sign North of Exit 74, Bolt w/ Flat Washers

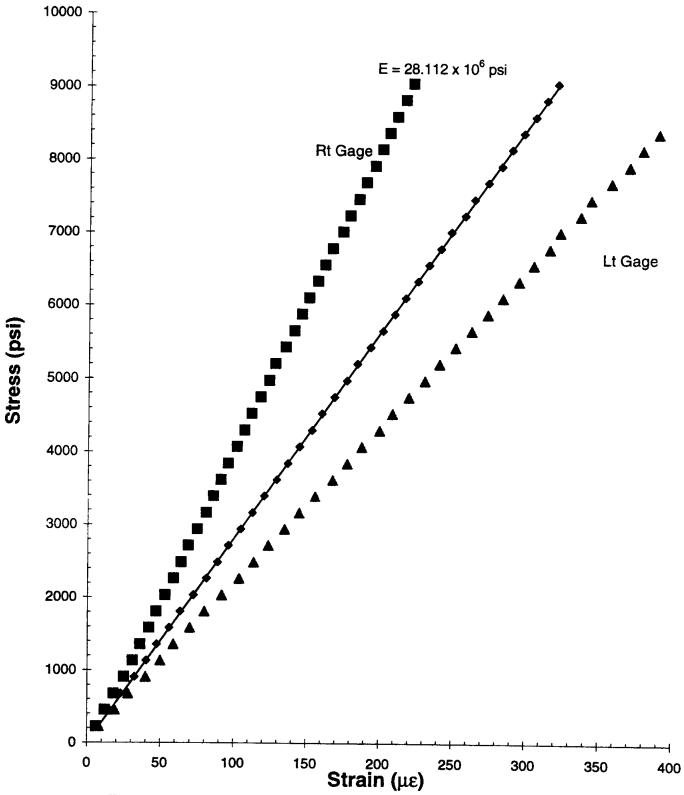


Figure 11 Stress vs Strain, Brevard (2) Sign, Bolt with Flat Washers, November 10, 1998

Stress vs Strain 2nd Brevard Sign North of Exit 74, Bolt w/ Flat Washers $E = 28.351 \times 10^6 \text{ psi}$ Rt Gage Stress (psi) Lt Gage

Strain (με)
Figure 12 Stress vs Strain, Brevard (2) Sign, Bolt with Flat Washers,
January 22, 1999

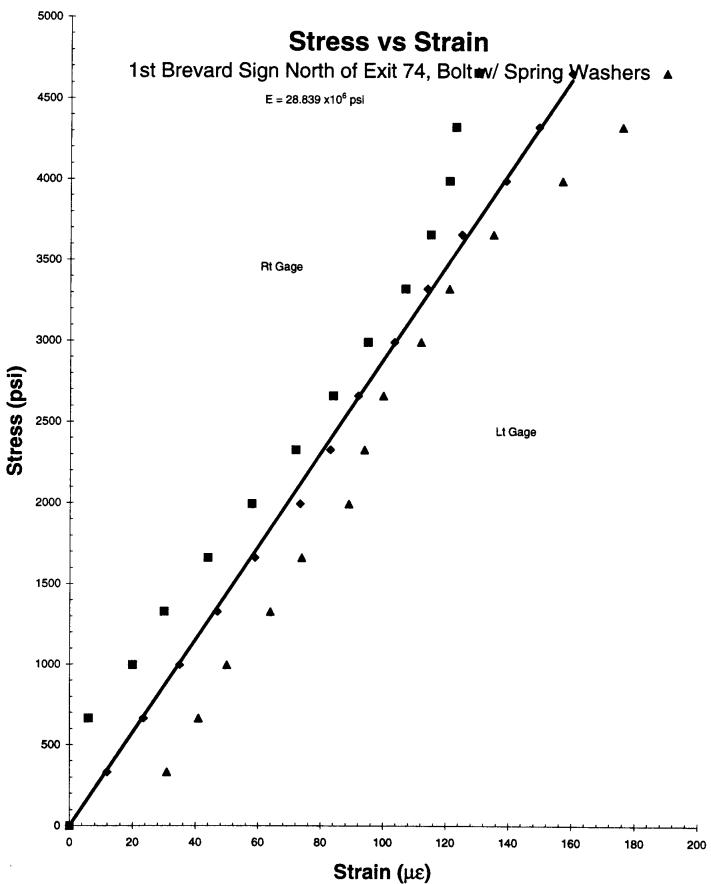


Figure 13 Stress vs Strain, Brevard (1) Sign, Bolt with Spring Washers, July 14, 1998

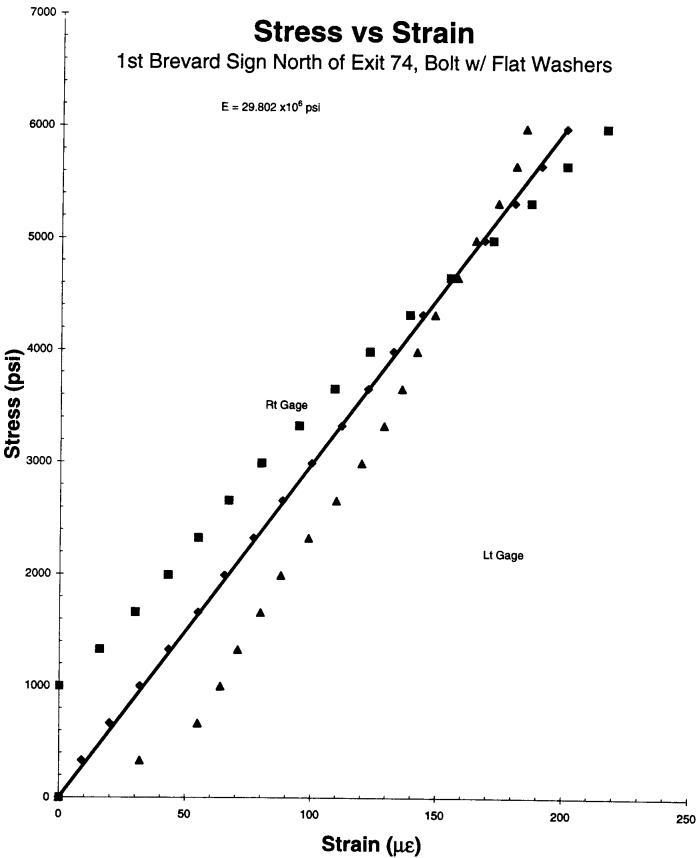


Figure 14 Stress vs Strain, Brevard (1) Sign, Bolt with Flat Washers, July 14, 1998

1st Brevard Sign North of Exit 74, Bolt w/ Spring Washers

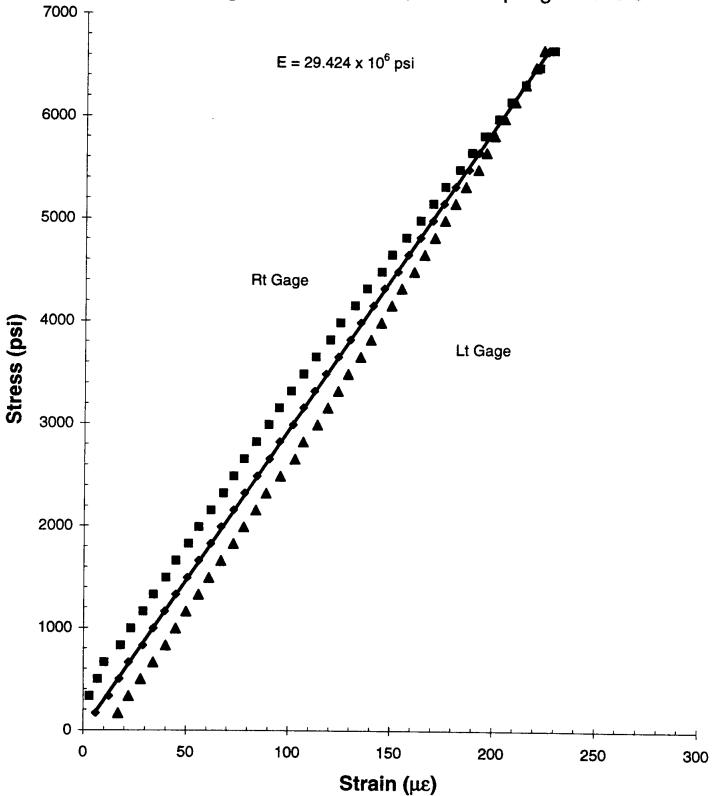


Figure 15 Stress vs Strain, Brevard (1) Sign, Bolt with Spring Washers, November 19, 1998

Stress vs StrainSebastian Sign, Bolt w/ Spring Washers

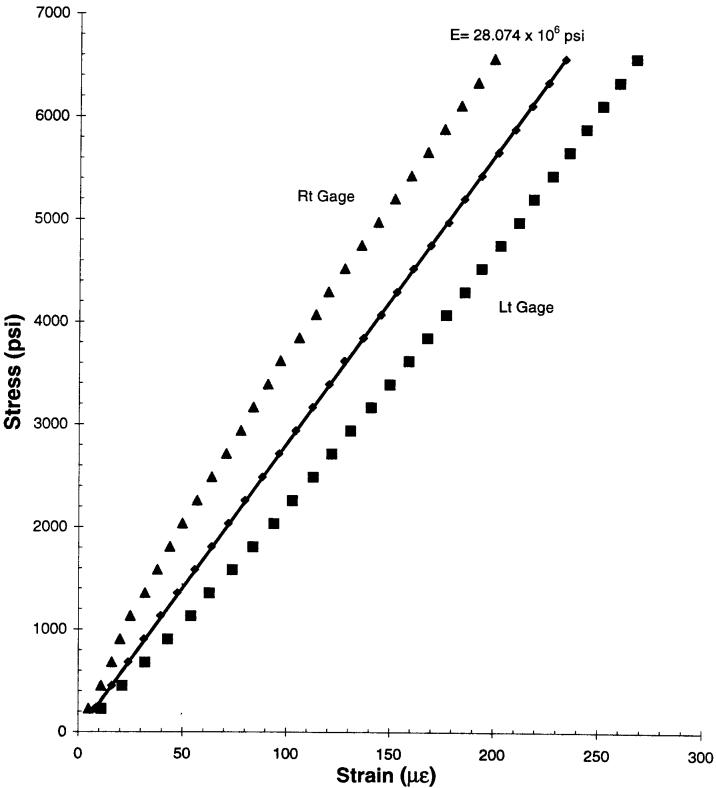


Figure 16 Stress vs Strain, Sebastian Inlet Sign, Bolt with Spring Washers, December 8, 1998

Stress vs Strain Sebastian Sign, Bolt w/ Flat Washers $E = 29.687 \times 10^6 \text{ psi}$ Rt Gage∎ Lt Gage Stress (psi) Strain (µE)

Figure 17 Stress vs Strain, Sebastian Inlet Sign, Bolt with Flat Washers, December 3, 1998

Stress vs Strain Indian River Sign, Bolt w/ Spring Washers $E = 28.899 \times 10^6 \text{ psi}$ Rt Gage Stress (psi) Lt Gage Strain (με)

Figure 18 Stress vs Strain, Indian River Sign, Bolt with Spring Washers, May 15, 1998

Indian River Sign, Bolt w/ Spring Washers

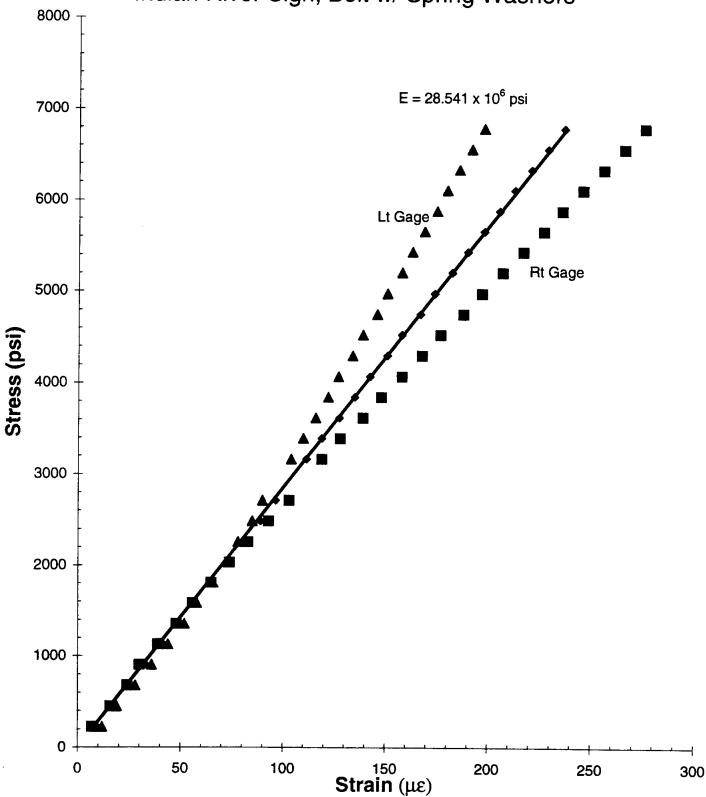


Figure 19 Stress vs Strain, Indian River Sign, Bolt with Spring Washers, February 3, 1999

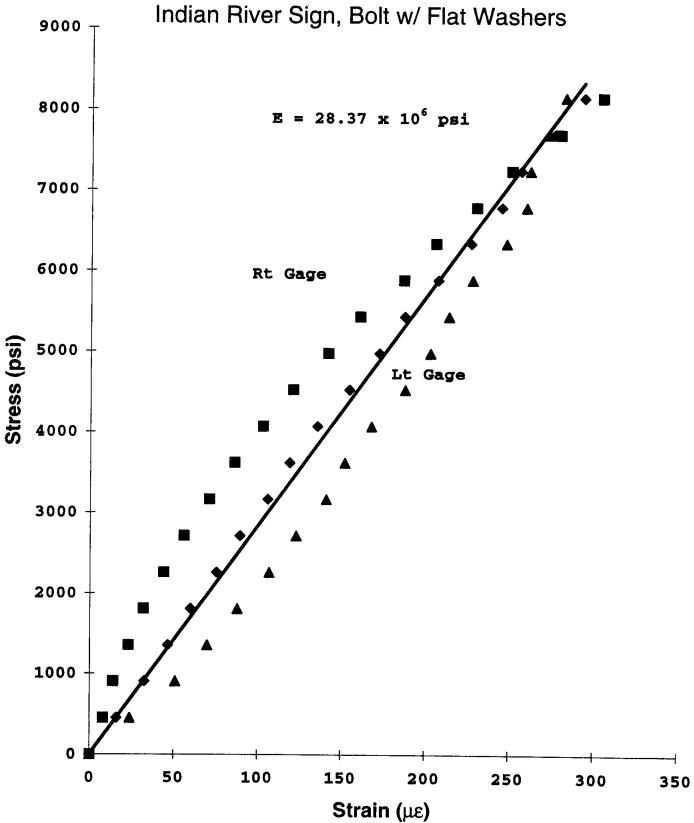


Figure 20 Stress vs Strain, Indian River Sign, Bolt with Flat Washers, May 15, 1998

Indian River Sign, Bolt w/ Flat Washers

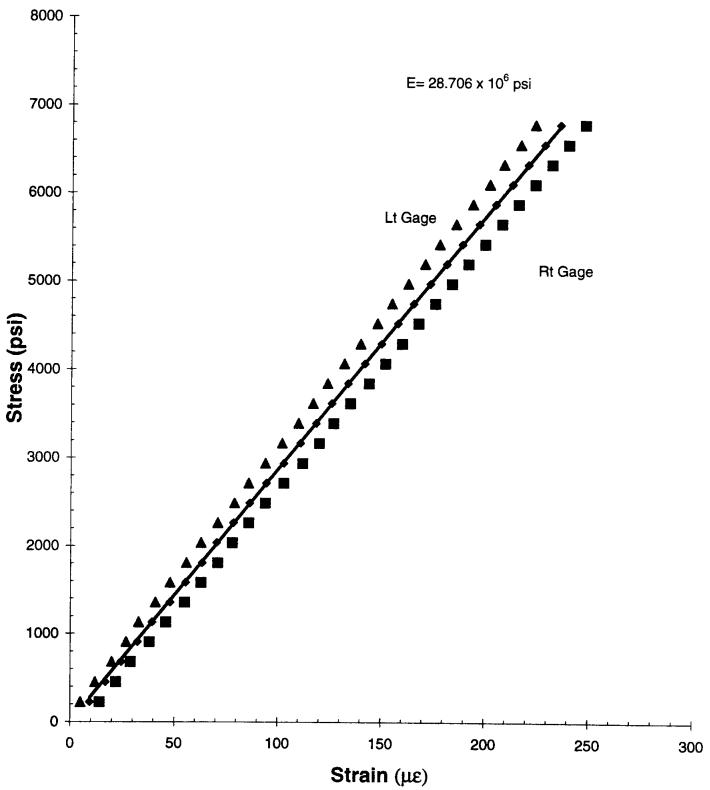


Figure 21 Stress vs Strain, Indian River Sign, Bolt with Flat Washers, December 8,1998

Stress vs Strain Indian River Bolt w/ Flat Washers

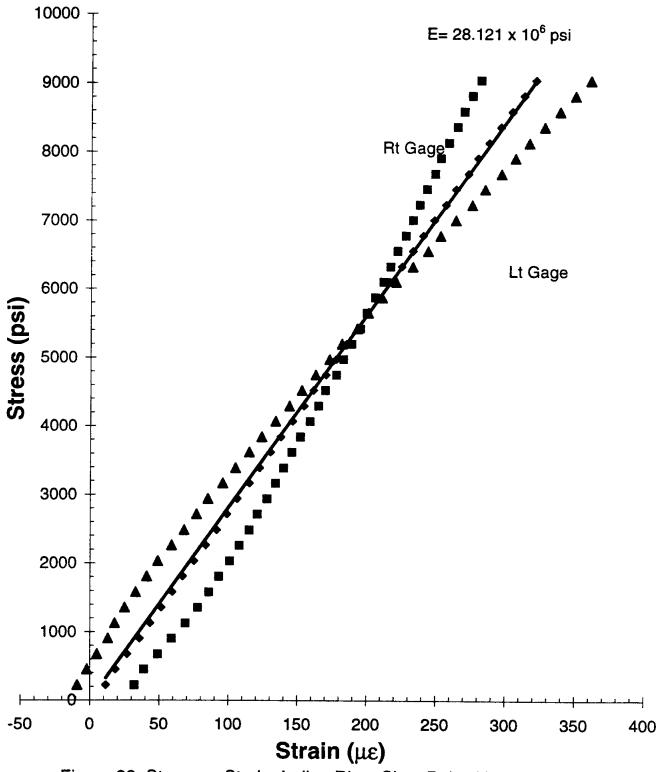


Figure 22 Stress vs Strain, Indian River Sign, Bolt with Flat Washer, February 2, 1999

Stress vs Strain Martin sign Bolt w/ Spring Washers $E = 28.664 \times 10^6 \text{ psi}$ Rt Gage Lt Gage Stress (psi) Strain (µE)

Figure 23 Stress vs Strain, Martin County Sign, Bolt with Spring Washers, December 3, 1998

Stress vs Strain Martin Sign, Bolt w/ Flat Washers

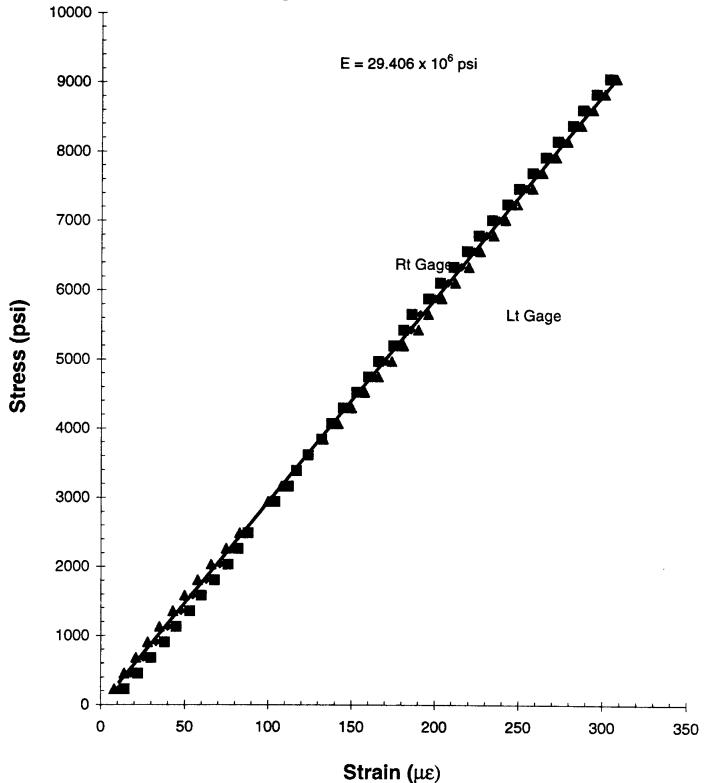


Figure 24 Stress vs Strain, Martin County Sign, Bolt with Flat Washers, December 3, 1998

RESULTS

The results of the field monitoring are presented below for each sign. In each case, the results are tabulated for the bolt with spring washers and the bolt with flat washers. In addition, the variation of tension over time is plotted for each bolt. The plots include also the variation of the temperature over time. Temperature values are represented by white circular dots, while the tension values are represented by black symbols.

Tampa

The Tampa sign bolts were installed on May 18, 1998 and they have been monitored ever since. Different persons have been taking readings ever since they were installed. The sign is located northbound along I-75 just before the I-75/I-4 junction. It is bolted down using 3/4"diameter bolts. Figure 26 shows the location of the sign with respect to I-4.

Tables 4 and 5 list the results of monitoring the bolts from May 18, 1998 to July 16, 1999. Figures 27 and 28 show a plot of tension vs. time for the bolt with spring washers and the bolt with flat washers.

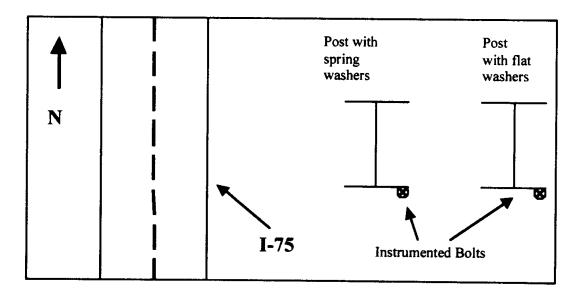


Figure 26 Location of the Tampa Sign with respect to I-75

After the initial installation, the bolt with spring washers show readings were below the lower limit of 2400 lb. The right gage readings also registered negative strains. We suspected a

defective spring washer stack so the bolt was re-torqued on June 10 with a new spring washer stack.

From June 24 to October 16, the readings were within the acceptable range as indicated by the trend line Figure 27.

The left wire was cut from the bolt head on October 23. The bolt was not replaced until February 2, 1999. One of the reasons why the bolt was not replaced for almost two months was that a FDOT crew was needed to assist in lifting the sign off the base. We also decided to focus on the other signs' bolts because most of them were damaged at that time.

The bolt was replaced on February 2, 1999 and the reading that day was in the acceptable range at 2755 lb. The reading increased to 4093 lb. the week after, on February 8. To date, the readings have kept within the acceptable range.

The bolt with flat washers was also installed on May 18, 1998. The tension readings on that day and on May 21 were 2915 lb. and 3613 lb. respectively. The tension readings increased to 4856 lb. and 4375 lb. on May 28 and June 1. The bolt was brought back to the lab to be re-tested and was re-installed on June 10. From June 18, 1998 to July 3, 1999 the tension readings were mostly in the acceptable range. The reading on October 3, 1998 was 865 lb. but this unusual reading was performed by a new research assistant and it could be disregarded. At the end of July, the gage was probably damaged and behaved erratically.

The tension in the short bolt could have been affected when the bolt with the spring washers was re-installed on February 1. A crane lifted the sign up on the post with the flat washers and this could have affected the tension. The bolt was re-torqued on February 8, 1999 and the tension reading that day was still low at 2159 lb. The tension reading decreased to 1787 on February 15 after which it increased to 2703 lb. on February 22.

Tampe; On I-75, south of I-4, sign on right hand when going north. Location:

"Plant City, Lakeland, Exit 53" Sign reads:

Green sign - 12' X 8' X 12' from ground, W8X24 posts Description:

Long Bolt

 Mores were dry and in the bag.
 Mores were dry and in the bag.
 More were dry and in the bag.
 More make dry and in the bag.
 LI where were dry and in the bag.
 LI where were dry and in the bag.
 Mos mot able to replace instrumented bott.
 Was not able to replace in the bott. bolt retorqued with new spring washer stack 0.021 bid 0.021 bid 0.024 bid 0.024 bid 0.027 0.102 0.101 0.084 0.083 0.022 0.039 0.055 12.4 14.6 11.5 12.3 13.0 Bolt tension KN 9.7 7.6 8.1 8.3 2755 4093 3024 3024 3036 2992 2992 2997 2947 2178 1806 1710 1813 1883 2780 2665 3280 2646 2582 2761 8 2921 Average 2 217 2 8 8 8 8 228 Strain Reading (µc) 유유우 150 8 2 8 ଷ 8 2 8 294 294 294 298 293 278 293 278 332 324 3328 343 352 352 352 Balance Offse 888888888 8 8 8 8 8 8 8 8 8 ક્ર श्च સ Tae 12:30 12:30 7:30 1:00 1:30 00:1 04:1 25:50 25:00 5:30 1:07 Date # of Days 5/16/98 148 155 175 182 182 182 183 194 304 304 465 465 465 465 465 465 ლ 2 13 22 86 33 888 1071698 117398 1117398 1117398 1117398 271599 272299 372299 475299 47589 47589 47589 47589 77589 77199 5/18/98 5/21/98 5/28/98 6/18/98 6/24/98 7/23/98 8/11/98 8/26/98 6/1/98 6/10/98 10/3/98 10/9/98 Operator ¥ ¥ SON ™ 5

Table 4 Tension and Eccentricity Values For Tampa Sign, Bolt with Spring Washers

Location: Tampa; On I-75, south of I-4, sign on right hand when going north.

Sign reads: "Plant City, Laketand, Exit 53"

Description: Green sign - 12' X 8' X 12' from ground, W8X24 posts

Short Boll

| 191 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2005 | 2 Eccent. (in) £ Bolt tension \$ Right Average Strain Reading (µc) Left Balance Offse (-2X) 894 Time Temp 8888 8888 Date # of Days 6/16/96 22 6/16/96 30 6/24/96 36 8/1/26/96 65 8/1/26/96 135 10/23/96 135 10/23/96 141 10/16/96 145 11/1/39/8 165 11/1/39/8 165 2/16/96 253 2/16/96 254 2/22/96 324 4/19/96 324 4/19/96 324 4/19/96 324 4/19/96 324 4/19/96 373 6/15/96 405 57.678 ₽ 5/21/98 5/28/98 5/18/98 6/1/98 Operator SON RE RE S S S S Z Z 5 5 5 5 5 5 5 न दे दे दे दे दे

Table 5 Tension and Eccentricity Values For Tampa Sign, Bolt with Flat Washers

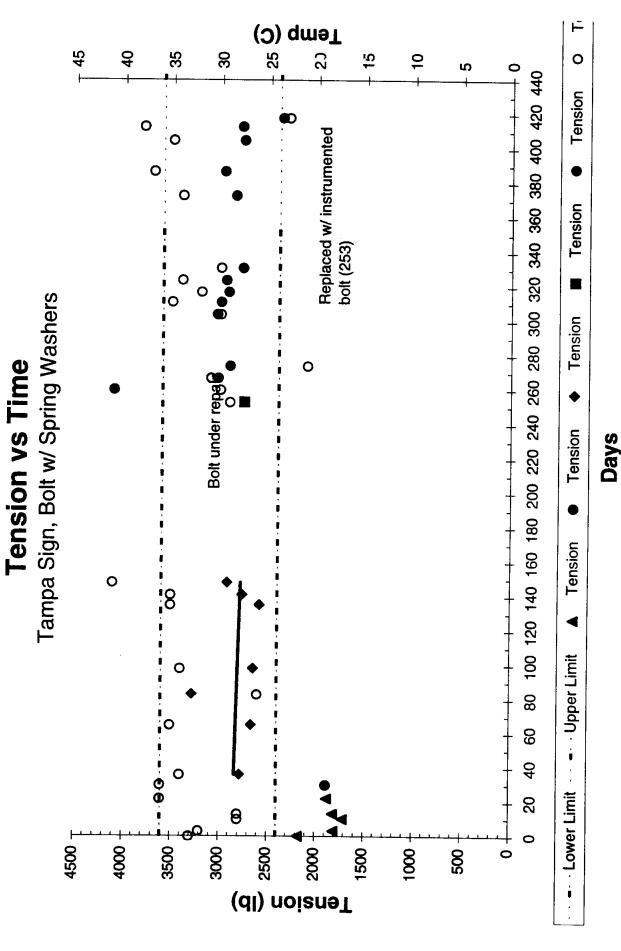


Figure 27 Tension vs Time, Tampa Sign, Bolt with Spring Washers

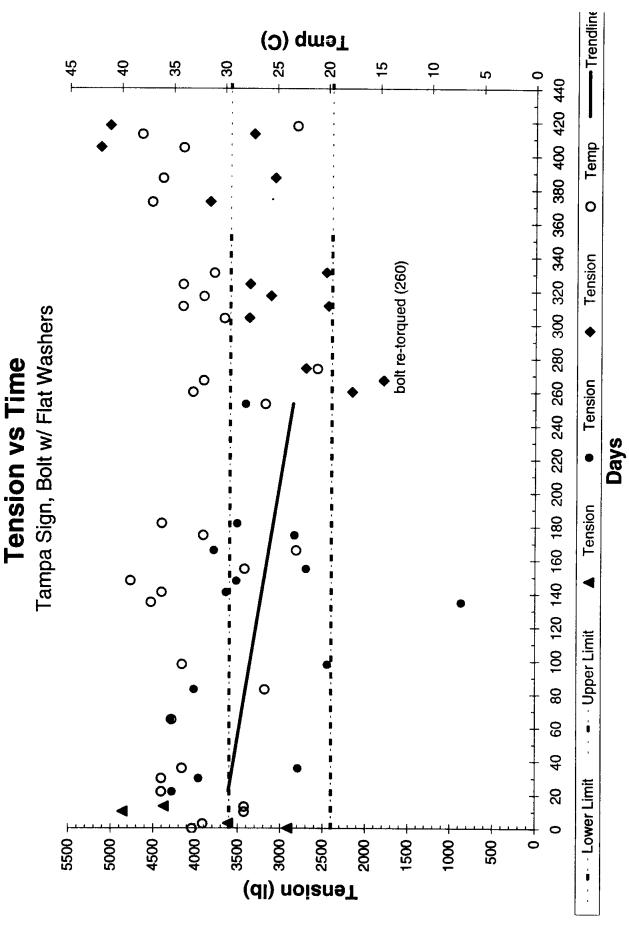


Figure 28 Tension vs Time, Tampa Sign, Bolt with Flat Washers

Orlando

The Orlando sign bolts were installed on May 16, 1998 and they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located westward along I-4, just before the Highway 528 (Beeline)/I-4 junction. It is bolted down using 3/4" diameter bolts. Figure 29 shows the location of the sign with respect to I-4.

Tables 6 and 7 list the results of monitoring the bolts from May 16, 1998 to April 12, 1999. Figures 30 and 31 show a plot of tension vs. time for the bolt with the spring washers and the bolt with flat washers.

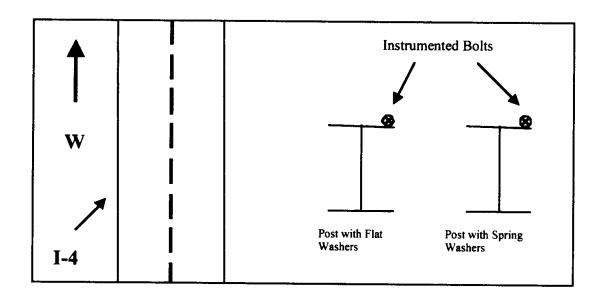


Figure 29 Location of Orlando Sign with respect to I-4

The bolt with strain gages was never replaced and the bolt was re-torqued. The spring washers were able to keep the tension within the acceptable range from May 16, 1998 to February 22, 1999. After that date, the readings were below the allowable range. On April 12, the bolt was removed. A tensile test in the lab yielded a value of 27,117 ksi for the modulus of elasticity, slightly below the expected value of 29,000 ksi, which might indicate a deterioration of the strain gage.

The initial reading for the bolt with flat washers was 2601 lb. The readings increased dramatically to 23420 lb. on May 28 and 45008 lb. on June 1. The bolt was brought back and reinstrumented. The bolt was re-installed on June 10 and a reading of 2671 lb. was recorded.

After that date, the readings were below 2400 lb. until September 26. The strain gage wires were ripped off on October 3 and the newly re-instrumented bolt was re-installed on November 4. The bolt was re-torqued on November 13 and the readings decreased to 922 lb. on November 20.

The bolt was re-installed on December 2, 1998 with new offsets and a measured modulus of elasticity of 27,761 ksi. The readings have been below the 2400 lb. until January 25, 1999.

The bolt was re-installed and re-aligned on February 1, 1999. One can notice that one of the strain readings was always negative for most of the readings. This meant that the bolt was not aligned properly and as a result, the bolt was subjected to bending. This is the reason for the retorquing of the bolt on February 8, 15, and 22. The bolt was finally removed and brought back to the lab. A tensile test revealed a modulus of elasticity of 36,047 ksi, a 33% increase over the value measured in January. This indicate a defect in the strain gage, probably in the bond between gage and bolt. This defect explains partially the erratic behavior of the bolt during the last weeks of testing.

Orlando, Last sign before exit 28 going west. Location:

"Exit 28, 1-4, Toff 528, Cape Canaveral, Titusvifle." Sign reads: Green Sign - 20' X 12' X 11' from ground, W8X24 posts Description:

Long Bolt										
Operator	Date	# of Days	Time	Temp	Stra	Strain Reading (µc)	(ant)	Bolt to	Bolt tension	Eco
		After		ပ္	Left	Right	Average	şqi	Ž.	٤
		5/16/96	Balance	Balance Offset >>	382	480				L
ML	5/16/98	0		54	218	310	264	3382	15.0	0.0
ML	5/21/98	5	•	58	271	586	285	3651	16.2	9
ML	5/28/98	12	•	58	248	315	282	3607	16.0	0.0
SON	6/1/98	15		82	237	325	281	3600	16.0	0.0
NOS	6/10/98	24	12:00	96	528	335	282	3613	16.1	0.0
NOS	6/18/98	32	11:20	32	218	288	253	3241	14.4	0.0
NOS	6/24/98	98	10:35	જ	500	313	257	3286	14.6	9.
NOS	96/8/2	52	10:50	33	508	278	243	3113	13.8	0.0
SON	7/15/98	29	10:30	88	171	315	246	3152	14.0	S
NOS	7/23/98	67	11:10	38	178	1 8	253	3235	14.4	9
Z	8/12/98	98	8:00	53	192	306	249	3184	14.2	9
z	8/26/98	100	11:30	33	178	315	247	3158	14.0	9
E	000000		10.01		***	250	,,,,	,,,,,		ľ

rusting on all washers told 12.7 12.8 13.2 13.1 241 180 148 166 190 225 172 178 180 187 197 173 88888 11:35 11:40 11:50 11:00 11:00 10:05 10:05 10:05 10:00 12:40 12:40 11:15 20 0 0 0 20 0 0 0 20 0 0 0 0 10/23/98 11/4/98 11/20/98 12/2/98 10/16/98 12/11/98 1/25/99 10/9/98

Table 6 Tension and Eccentricity Values For Orlando Sign, Bolt with Spring Washers

readings fluctuated; replaced w/ regular bolt permanently

Location: Orlando, Last sign before exit 28 going west.

Sign reads: "Extt 28, 1-4, Toll 528, Cape Canaveral, Thusville."

Description: Green Sign - 20' X 12' X 11' from ground, W8X24 posts

Short Boft

						boft tested in lab and retorqued								"check twice, it is correct	Strain gage wires were indoed off from the bolts.	Replaced instrumented both with requiser both.	Have not replaced regular bolt with an instrumented bolt	Replaced regular both with newly received instrumented bott. Did not read Termo.	Re-torqued bolt. New Offsets: £1:135 Rt: (-2k)719. Wires were dry and in beg.	Wires were dry and in the beg.	Average was very low. Replaced with a regular bolt.	Replaced w Instrumented bolt. New Offsets: Lt. 114 Rt. (-2k)722. New E = 27.761x106kps!			Whres were dry and in the bag.		3	***************************************	Re-installed & re-aligned the bolt. New Offsets Lt: 120 Rt (-2k)694	Retorqued winew wrench. New Offsets Lt. 250 Rt (-2k)678	Retorqued, initial tension ridgs were negative. Lt:345 Pt:/-2k1781	Retorqued, Lt:418 Rt:(-2k)774	Whee were dry and in the bag.		200	0.166 Transferos fluctuated; rankaced w/ remitar hot nermanenthy
Eccent.	£		0.075	0.044	0.076	0.084	0.237	0.287	0.287	0.307	0.341	0.367	0.318	90.0				0.012		0.147	0.073	0.189	0.209	0.212	0.266		0.208	0.312		0.243	0.267	0.198		-0.232	0.256	0.166
Bolt tension	ĸ		11.6	43.2	104.2	200.2	11.9	8.9	8.7	7.7	6.7	62	7.0	7.8				9.5	6.3	5.5	4.1	10.7	8.8	8.6	6.8	7.6	7.6	4.6	9.6	7.6	4.9	11.9	-0.8	-5.4	4,1	7.1
Bolt te	s q		2601	9706	23420	45008	2671	1992	1954	1723	1505	1384	1582	1762				2140	1409	1243	922	2404	1987	1944	1533	1717	1705	1036	2208	1705	1098	2674	-172	-1220	920	1594
(mc)	Average		203	758	1828	3513	508	156	153	135	118	801	124	138				167	110	26	7.2	196	162	159	125	140	139	84.5	180	139	89.5	218	-14	-99.5	7.5	56
Strain Reading (µe)	Right	(-2k)774	366	399	343	356	-319	-320	-314	-306	-310	-315	-295	0				145	430	249	16	-200	-200	-200	-230	-170	-170	-197	-140	200	-165	-242	-271	-346	-130	9
Stra	Left	418	40	1116	3313	0299	736	631	619	575	545	531	542	275				189	-210	-55	128	592	524	517	480	450	448	366	200	-222	344	678	243	147	280	88
Temp	ပ္	Offset >>	54	58	53	53	36	32	35	33	28	38	29	33	96	35			25	29	96	25	28	27	21	29	56	26	21	22	21	12	23	31	28	23
Time		Balance Offset			-		12:00	11:20	10:35	10:50	10:30	11:10	8:00	11:30	12:41	11:50	11:35	12:10	12:05	12:05	11:10	10:15	1:10	2:10	10:10	12:40	1:40	11:25	1:10	10:55	10:35	10:25	10:25	10:36	10:00	5 5 5
# of Days	After	5/16/96	0	5	12	15	24	æ	38	52	28	29	88	100	137	143	150	157	168	177	184	96	196	196	205	205	302	249	255	292	569	276	906	313	319	88
Date			5/16/98	5/21/98	5/28/98	6/1/98	6/10/98	6/18/96	6/24/98	7/8/98	7/15/98	7/23/98	8/12/98	8/26/98	10/3/98	10/9/98	10/16/98	10/23/98	11/4/98	11/13/98	11/20/98	12/2/98			12/11/98			1/25/99	2/1/99	2/8/99	2/15/99	2/22/99	3/22/99	3/29/99	4/5/99	4/12/99
Operator			¥	ML	Æ	SON	NOS	NOS	SQN	SON	SON	SON	z	z	75	5	Ľ	T.	5	5	5	5			5			Ŧ,	5	Ţ	Ţ	<u>ل</u>	۲,	+5	5	F,

Table 7 Tension and Eccentricity Values For Orlando Sign, Bolt with Flat Washers

Tension vs Time

Orlando Sign, Bolt w/ Spring Washers

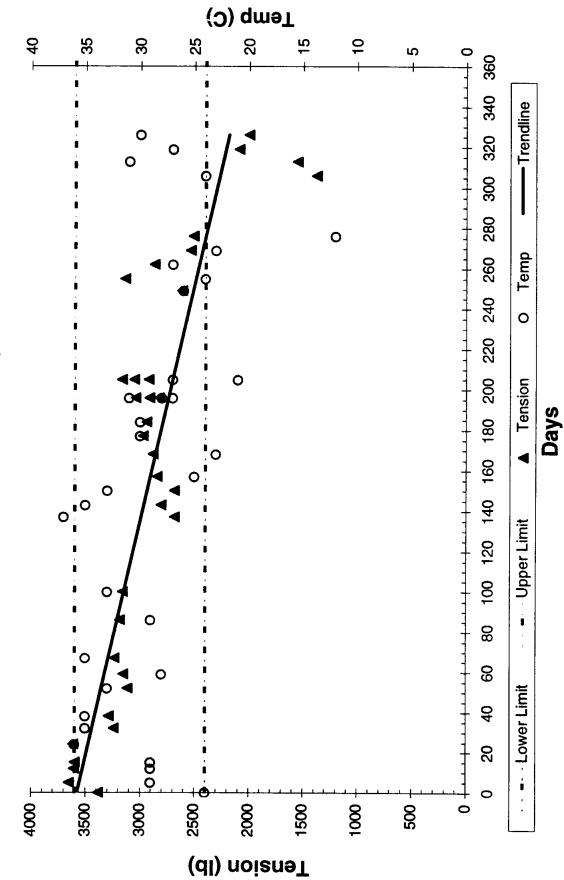


Figure 30 Tension vs Time, Orlando Sign, Bolt with Spring Washers

Tension vs Time

Orlando Sign, Bolt w/ Flat Washers

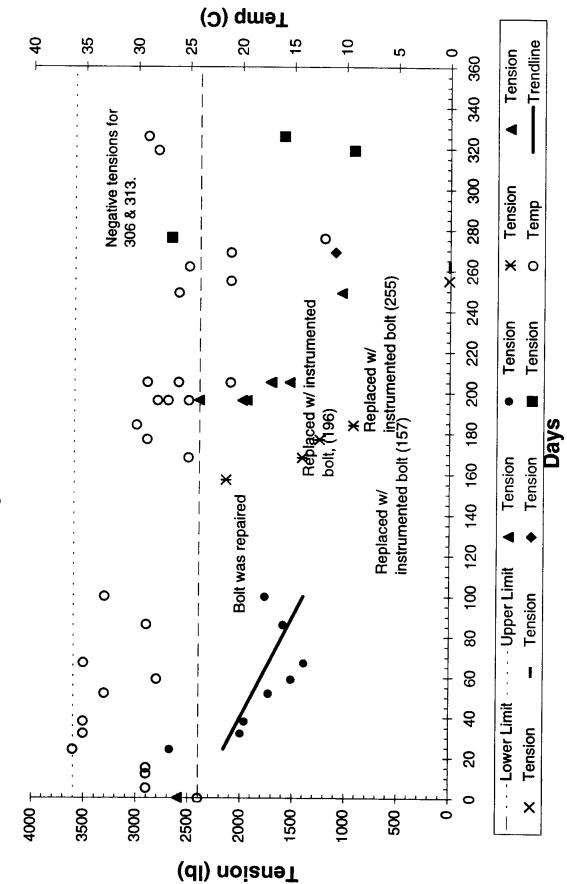


Figure 31 Tension vs Time, Orlando Sign, Bolt with Flat Washers

Brevard (2)

The Brevard (2) sign bolts were installed on May 16, 1998 and they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located northbound along I-95, past Exit 74. It is bolted down using 3/4" diameter bolts. Figure 32 shows the location of the sign with respect to I-95.

Tables 8 and 9 list the results of monitoring the bolts from May 16, 1998 to April 19, 1999. Figures 33 and 34 show a plot of tension vs. time for the bolt with the spring washers and the bolt with flat washers.

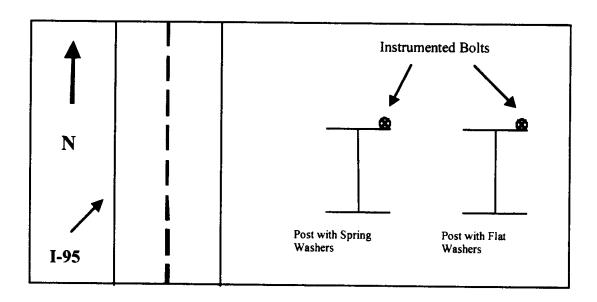


Figure 32 Location of Brevard (2) Sign with respect to I-95

The spring washers were able to hold the tension within the acceptable range from May 16, 1998 through November 20, 1998 except for two days: October 23 at 1883 lb. and November 4 at 1838 lb. These readings might have been affected by the drop in temperature in those days.

Consequently, multiple readings were done for two days to test whether the temperature had any effects on the strain gages. The readings taken on those two days, December 2 and 11, seemed to indicate that the tension varied with the temperature. However, temperature tests performed in the lab indicated that the temperature should have no effect on the strain gages. Thus, it was concluded that the gages or the adhesive might have some defects. The bolt was removed and brought back to be re-instrumented on January 25, 1999.

The newly re-instrumented bolt was re-installed on February 2, 1999 with new offsets and with a measured modulus of elasticity of 28,634 ksi. The readings have kept within the acceptable range, until April 19, 1999, when the readings were discontinued.

The bolt with the flat washers was installed on May 16, 1998. The reading on that day was 3523 lb., within the acceptable range. The readings increased to a peak of 6739 lb. on May 28. The bolt was taken back to the lab and it was re-installed on June 1 where the reading was 6009 lb.

On June 10, the bolt was re-installed after the bolt was brought back to the lab to have the lead wires resoldered. Readings were taken from June 24 to September 26 which saw the readings fluctuate from a low of 1557 lb. to a high of 6239 lb. The bolt was submerged in water for at least a day, on September 10.

From October 3 to October 23, 1998, the right gage registered and infinite resistance on the Strain Gage Tester which meant that it was disconnected. The bolt was removed on November 3 and was re-installed on November 13 with a measured modulus of elasticity of 28,112 ksi. After that date, the readings increased dramatically to 11060 lb. and 26565 lb. on December 2, 1998. The bolt was replaced and re-installed on January 25, 1999 with a modulus of elasticity of 28,112 ksi. The reading on that day was low at 1165 lb. The following reading on February 1 was even lower at 777 lb. The bolt was re-torqued using a new torque wrench on February 8. The readings have been decreasing until April 19, 1999, when the readings were discontinued. A tensile test in the lab showed a value of 28,115 ksi for the modulus of elasticity, with no change from the value measured before installation in the field. The gage was then judged to be sound.

Location: Brevard; second sign past exit 74 going north.

Sign reads: "Cocoa, Orlando, Next 2 exit."

Description: Green Sign - 12' X 8' X 15' from ground, W8X24 posts

Long Bolt
Operator Date # of Dava

										-				wires ripped off duct tape storn	Wires ripped off duct tape and sitting in water for cultie a while	Botts still under water					wires were in bad	With word of the han	Wires were div and in the ban	With worth and in the han Distington of these weekers Described Aprel 201	Wites were dry and in the bad. Rolls were wet		Whee were dry and in the bag.				:	Wires were dry and in the bag.		Ranisonal inethernanted both of receipts both	Still instrumention & besting hot	reinstalled instrumented both Offsets 1: 1-24/716 Bt. 1.24/899 F. 29 69/41064-24	wires were dry and in bad	Pig.	rusting on all washers	PiQ	posts were wet, sign in shadows	wires were in bag	replaced w/ regular bott permanently
Eccent.	3		0.035	0.003	0.003	0.003	0.00	0.00	0.022	0.025	0.033	0000	0.034	0.033			9000	0.045	0.048	0.034	0.047	0.049	0.046	0.037	0.036	0.047	0.053	0.045	0.052	0.043	0.0	9000	38	2		0.022	0.024	0.028	0.023	120.0	0.021	0.023	0.022
mston	¥		12.4	14.7	14.5	14.2	12.4	13.0	12.7	14.0	13.9	14.3	10.2	14.5			14.1	14.6	13.9	12.7	14.7	13.8	13.3	8.4	8.2	14.9	15.2	15.5	11.8	8.8	S. 5	13.7	000	2		14.8	13.2	11.7	13.4	=	13.4	13.7	13.3
Bolt tension	٥		2787	3299	3254	3190	2780	2915	2863	3139	3126	3222	2283	3254			3165	3293	3126	2857	3312	3113	2979	1883	1838	3357	3414	3491	2646	1973	2000	2880	2242			3333	2960	2638	3017	3175	3023	3087	2998
(mc)	Average		218	528	254	249	217	877	224	245	244	252	179	254			247	257	244	223	528	243	233	147	4	262	267	273	23	<u> </u>		Ę	175		-	3 64	234	508	539	251	239	244	237
Strain Reading (uc)	Right	(-2k)839	137	249	262	241	227	237	277	310	330	333	114	343			347	379	368	98 4	387	369	347	202	198	392	418	\$	325	S	513	86	248			203	173	147	180	196	185	184	18
Stra	Le	(-2K) 716	298	566	246	257	207	218	170	180	158	170	244	165			147	135	120	142	130	117	118	68	88	52	15	₹	5	æ :	8 8	3 2	8			324	532	270	297	906	293	క్ల	293
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Time		Balance			•		10:45	10:15	9:30	9:45	8:40	10:00	1:10	10:30	10:18	5:01	10:05	6:45	10:25	2:50	11:10	10:23	10:20	10:25	10:40	10:45	9:50	8	12:00	85	35.0	1:40	3:25	10:00	3:10	10:30	9:15	00:8	9:02	9:15	959	9:10	9:38
# of Days	After	5/16/96	0	5	12	15	24	32	38	25	8	67	88	8	114	114	123	123	130	130	137	143	50	157	991	177	184	96	86	<u>8</u>	302	386	206	249	255	566	569	276	906	313	319	326	333
Date			5/16/98	5/21/98	5/28/98	6/1/98	6/10/98	6/18/98	6/24/98	2/8/86	2/16/98	7/23/98	8/11/98	8/26/98	9410498	9/10/98	9/19/98	9/19/98	9426/98	8426/88	10/3/98	10/9/98	10/16/98	10/23/98	11/4/98	11/13/98	11/20/98	12/2/96	+		12/11/98		-	1/25/99	2/1/39	2/12/99	2/15/99	2/22/99	3/22/99	3/29/99	4/5/99	4/12/99	4/19/99
Operator			¥	₹	₹	SON	SON	SON	NOS	SON	SON	SON	z	z	¥	Z.	M.	ž	¥	ž	5	<u>L</u> ,	Τſ	5	15	5	5	5	†	+	<u> </u>			ŢĊ	F5	<u> </u>	5	=	5	5	5	5	5

Table 8 Tension and Eccentricity Values For Brevard (2) Sign, Bolt with Spring Washers

Brevard; second sign past exit 74 going north. Location:

Sign reads: "Cocoa, Orlando, Next 2 exit."

Description: Green Sign - 12' X 8' X 15' from ground, W8X24 posts

Time Temp Strain Reading (µc)	rain Reading Right	in Reading (µc) Right Average	(µc) Average	-	Bolt te	Bolt tension	Eccent. (in)
Balance Offset >> 228 (-2k)844	Ť	(-2k)844		_			
. 27 157 393 275	393	_	275	\vdash	3523	15.7	0.040
- 29 415 477 446	477		446		5714	25.4	0.007
- 27 532 520 526	520		526	\vdash	6239	30.0	0.001
- 29 452 486 469	486		469	_	6009	26.7	0.003

Date

Operator

Short Bolt

5/16/98

¥ ¥

5/21/98 5/28/98 6/1/98 6/10/98

NOS NOS

boft removed, lead wire resoldered and bolt retorqued

18.6

whee were in the bag but were wet.

Right gage still registered an infinite resistance.

Whes were dry and in the bag. Right gage read an infinite resistance.

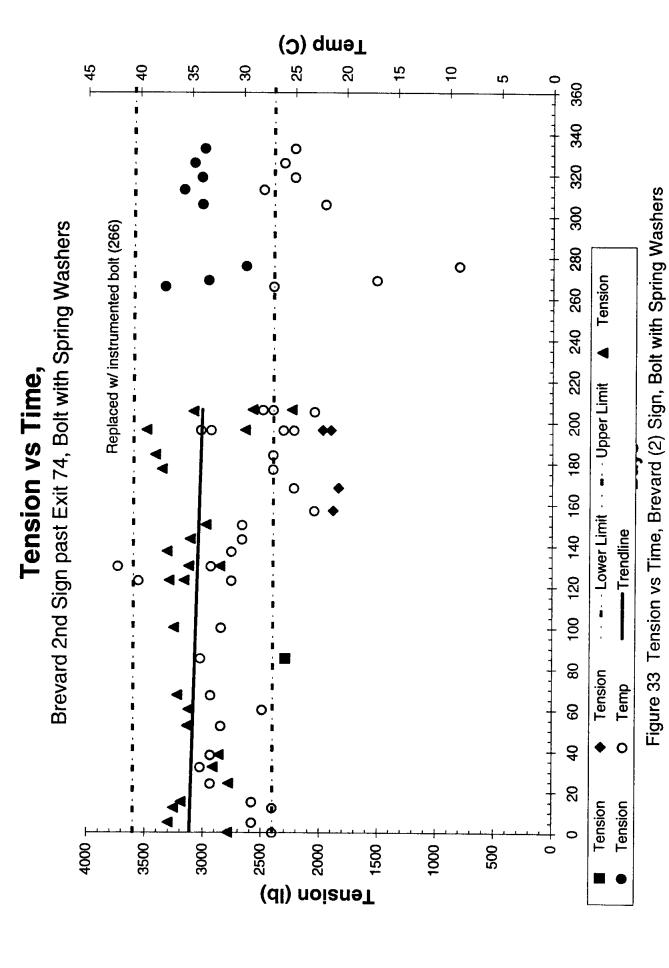
White out and spliced and still read an infinite resistance.

Replaced instrumended both with regular bott.

Replaced wi instrumented both. New Orsets: Lt: 274 fft: (-2k) 699 New E=28.112x10* kpsi.

Whres were dry and in the bag. Measured 2x. 0.015 Replaced w/ instrumented bolt. New Olsets: Lt: 228 Rt: (-2k) 844 New E=28.112x10*kpsi Whres ripped off duct tape and sitting in water for quite a white Botts still under water 0.057
0.090 Whres were dry and in bag
0.090 whres were dry and in bag
0.021 bbd
0.024 bbd
0.041 bbd
0.002 posts were wet, sign in shadows
0.013 whres were in the bag
0.007 Replaced w regular both permanently Will replace this next week. Replaced w/ regular bolt 14.8 0.034 R 23.5 0.043 W 49.2 0.070 118.2 0.083 W 0.024 0.005 0.007 0.018 0.022 0.023 6.9 21.9 27.8 25.8 21.3 23.4 16.2 20.6 21.1 15.7 3316 5291 11060 26565 3645 4638 4740 3536 4177 1557 4933 6239 4798 5253 1165 777 1547 1040 1046 1196 1196 1184 1128 326 122 385 385 487 487 453 410 388 267 426 891 2139 276 712 330 607 570 541 8 2 33 33 33 33 242 242 163 163 132 137 121 25 23 2 28 23 24 28 23 24 28385838 8888888 8 8 8 8 10.45 8:40 8:40 10.00 10.00 10.18 10.18 10.00 10.00 10.00 11.2 9:50 9:05 9:05 85.8 900 2/1/89 2/1/89 2/15/99 2/12/99 3/22/99 4/5/99 672498 77898 776998 87698 87698 87698 971998 971999 971999 971999 107398 107698 107698 117498 12/11/98 4/12/99

Table 9 Tension and Eccentricity Values For Brevard (2) Sign, Bolt with Flat Washers



Tension vs Time

2nd Brevard Sign North of Exit 74, Bolt w/ Flat Washers

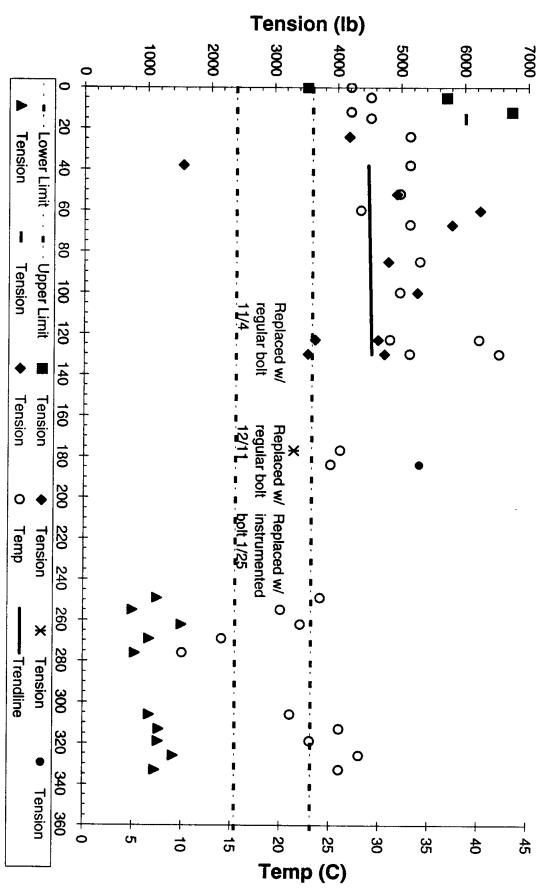


Figure 34 Tension vs Time, Brevard (2) Sign, Bolt with Flat Washers

Days

Brevard (1)

The Brevard 1 sign bolts were installed on July 14, 1998 and it they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located northbound along I-95, past Exit 74. It is bolted down using 7/8" diameter bolts. Figure 35 shows the location of the sign with respect to I-95.

Tables 10 and 11 list the results of monitoring the bolts from July 14, 1998 to July 15, 1999. Figures 36 and 37 show a plot a plot of tension vs. time for the bolt with the spring washers and the bolt with flat washers.

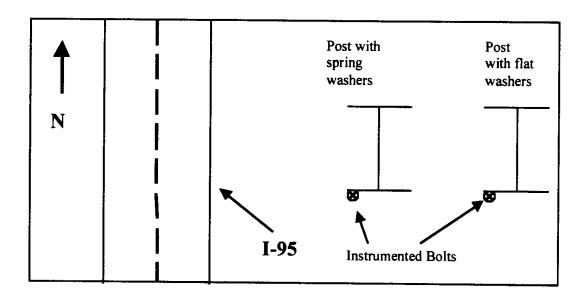


Figure 35 Location of Brevard (1) Sign with respect to I-95

The bolt with spring washers remained in the acceptable range from July 1998 to November 1998 except for one day: September 26 at 3915 lb. and 3749 lb.

The bolt was replaced on November 4, 1998 because the right strain gage became loose. The newly re-instrumented bolt was re-installed on November 20, 1998 along with a new spring washer stack. After that date, the readings were initially above the upper limit of 3600 lb. until February 15, 1999. From then on, the reading were in the allowable range. On June 15, 1999, the wires were found cut. Therefore the readings were interupted.

The bolt with flat washers was also installed on July 14, 1998. The washers were only able to hold the tension within the acceptable range on July 14 and on July 16. Since then, the readings have always been below the allowable range with a substantial reduction in tension of the bolt. One can notice that negative strain readings were periodically recorded which is reflected in the relatively large eccentricities of the tensions.

The bolt was submerged in water once on February 1, 1999 but it was still possible to get readings that day.

Location: Brevard; first sign past exit 74 going north.

Sign reads: "Highway 520 - 4 miles, Jacksonville - 151 miles"

Description: Green Sign - 19-6" X 6" X 21" from ground, W10X45 posts, 7/8" bolts

Whres were dry and in the bag. Right gage read an infinite resistance.
Cut and spiked both wires. Ri gage read infinite resistance. Will replace next week.
Replaced instrumented both wit regular both.
Sitil repealing the both.
Replaced with new instrumented both and new spring washer stack. 0.065
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0.066 wires left out of the bag & exposed Wires were in the bag and dry. 900.0 0.076 0.067 0.063 0.045 0.025 0.017 0.074 (in) 15.7 15.4 **Bolt tension** 3113 3305 3523 3915 3749 3313 \$q 2956 82 Average 170 98 Strain Reading (µe) **55** 22 25 125 125 135 136 136 12 22 57 156 176 176 221 392 352 284 338 237 265 267 267 267 268 259 259 888= 88888 ଷ 10:30 9/19/98 9/19/98 9/26/98 10/3/98 10/16/98 11/13/98 11/13/98 2722/99 3723/99 4/5/99 4/12/99 4/13/99 Date 7/14/98 7/16/98 7/23/98 8/11/98 8/26/98 12/11/98 Long Bolt Operator * * * * 5 5 5

Table 10 Tension and Eccentricity Values For Brevard (1) Sign, Bolt with Spring Washers

wires were cut

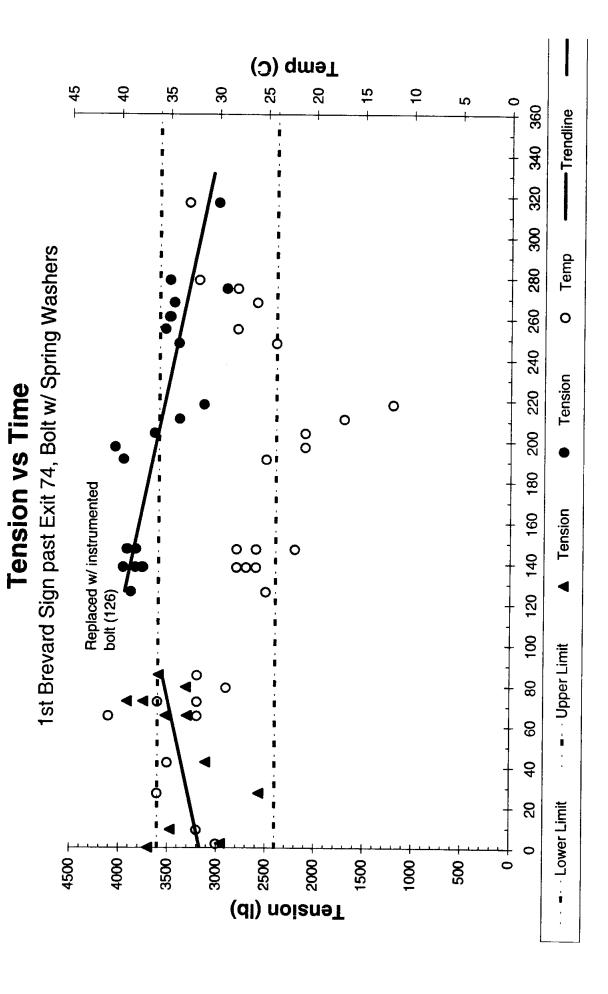
Location: Brevard; first sign past exit 74 going north.

Sign reads: "Highway 520 - 4 miles, Jacksonville - 151 miles"

Description: Green Sign - 19-6" X 6' X 21' from ground, W10X45 posts, 7/8" bolts

							*check twice, it is correct					wires were kept in the plastic bac	Wires were dry and in the ban	Wires were div and in the han. Both reactions fluctuated a lot	Wires were dry and in the han	Wines were dry and in the had. Boths were west		Wites were dry and in the han	, n							Wires were dry and in the bad.	bolls were submerced in water	Wires were dry and in the bag.	•								values fluctuated with	(
_		т-	1	_	<u> </u>	ī	-check	T-	-	_	_	¥ uses ¥	Wires *	W res	Wines	Wines		¥ Wies ≰	8	_	Τ	_	2	3	2	Wires	DOMS WIB	Wires w	3	2	Đ	3	3	Z	8	<u>2</u>	Values fi			
Eccent.	£		0.133	0.147	0.151	0.205	0.080	0600	0.087	0.100	0. 135	0.114	0.132	0.121	41.0	0.142	0.118	0.102	0.109	0.092	0.097	0.093	0.079	0.071	0.072	060.0	0.068	0.081	0.166	0.121	0.103	0.089	0.084	0.063	990.0	0.054	0.023	0.062	0.050	0.049
Bolt tension	Ž		11.8	11.2	9.5	6.5	9.1	8.4	8.8	7.3	7.0	7.0	5.4	5.0	4.8	5.2	5.6	7.1	5.9	5.9	5.6	6.1	5.7	6.1	5.5	0.4	5.3	5.5	3.6	2.9	5.5	6.2	5.7	4.3	6.3	7.1	7.9	8.4	8.6	6.4
Bolt te	ĝ		2651	2529	2145	1456	2049	1892	1988	1648	1569	1569	1203	1125	1090	1177	1256	1587	1317	1334	1264	1369	1273	1378	1229	206	1195	1229	802	645	1238	1395	1273	426	1413	1587	1779	1892	1927	1447
(ang	Average		152	145	123	æ	118	109	114	8	06	8	66	28	83	88	72	91	9/	11	23	62	7.3	79	71	52	69	7.1	46	37	7.1	8	73	98	18	16	102	801	E	83
Strain Reading (µe)	Right	399	337	340	293	240	203	198	205	181	176	184	152	136	145	155	150	176	151	14	137	145	126	130	117	96	111	123	116	78	138	145	129	88	130	136	123	99	161	120
Strai	Left	374	÷33	Ģ.	47	-73	38	19	23	8	•	4	-14	-2	-20	នុ	چ	6	٥	12	8	12	50	58	24	6	92	18	-24	•	4	15	=	24	35	46	8	22	8	46
Temp	္	Balance Offset >>	•	30	æ	98	88	7	35	32	96	53	90	31	21	22	52	25	23	27	26	25	22	27	25	22		21	15		22	56	58	82	22	5.8	33	30	8	ક
Time		Balance		9:00	9:45	1:00	10:30	10:25	7:10	10:45	3:10	10:52	10:05	10:05	9:51	10:15	10:25	8:55	8:40	11:35	3:15	4:15	8:40	11:30	3:06	9:25	3:00	8:45	8:40	8:36	8:40	00:6	9:30	8:50	9:15	10:15	11:30	11:30	1:15	10:10
# of Days	After	7/14/96	0	2	6	27	42	99	99	72	72	7.9	88	85	86	110	119	126	138	138	138	138	147	147	147	181	187	Ř	211	218	248	255	261	268	275	279	317	331	348	361
Date			7/14/98	7/16/98	7/23/98	8/11/98	8/26/98	9/19/98	9/19/98	9/26/98	96/92/6	10/3/98	10/9/98	10/16/98	10/23/98	11/4/98	11/13/98	11/20/98	12/2/98				12/11/98			1/25/99	2/1/89	2/8/39	2/15/99	2/22/99	3/22/99	3/29/89	4/5/99	4/12/99	4/19/99	4/23/89	5/31/99	6/15/99	7/2/99	7/15/99
Operator			NOS	SON	NOS	z	z	ML	¥	ž	ž	5	5	J.	٦.	Τſ	JT	Τſ	٦,				5			5	±°	F.	5	5	5	5	Τc	Τſ	5	5	g G	ddf	JPP.	del

Table 11 Tension and Eccentricity Values For Brevard (1) Sign, Bolt with Flat Washers



Days Figure 36 Tension vs Time, Brevard (1) Sign, Bolt with Spring Washers

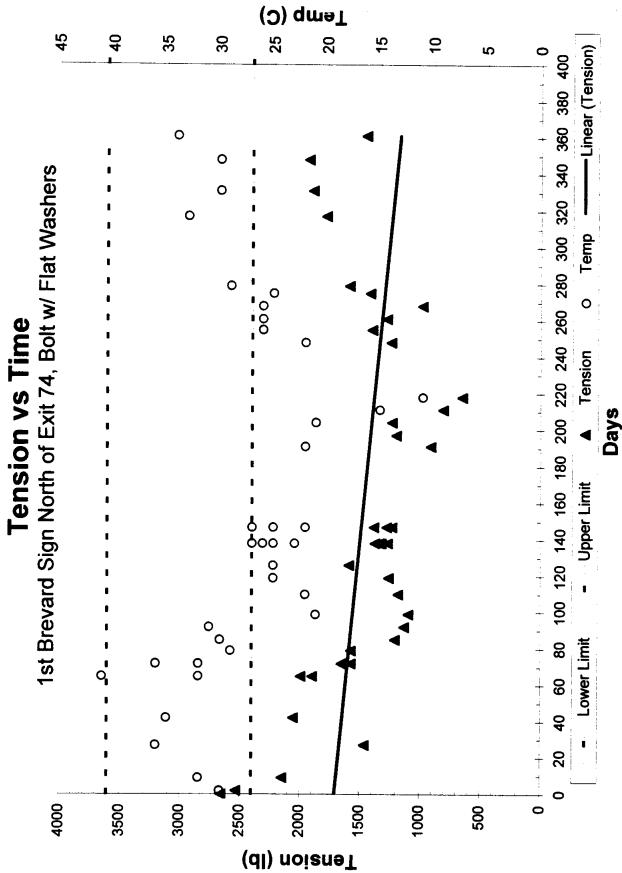


Figure 37 Tension vs Time, Brevard (1) Sign, Bolt with Flat Washers

Sebastian Inlet

The Sebastian Inlet sign bolts were installed on May 15, 1998 and they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located northbound along A1A just south of the Sebastian Inlet Bridge. It is bolted down using 3/4" diameter bolts. Figure 38 shows the location of the sign with respect to A1A.

Tables 12 and 13 list the results of monitoring the bolts from May 15, 1998 until June 11, 1999. Figures 41 and 42 show a plot of the tension vs. time the bolt with the spring washers and the bolt with the flat washers.

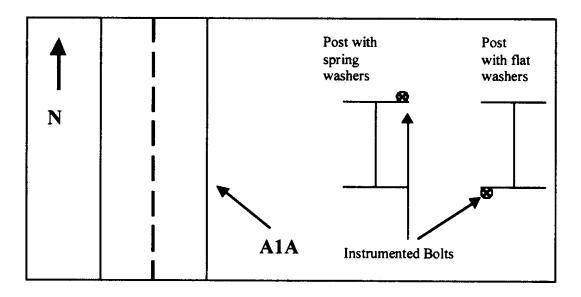


Figure 38 Location of Sebastian Inlet Sign with respect to A1A

The Sebastian Inlet bolt with spring washers was able to maintain the tension within the acceptable range until June 25 with only one reading dipping below the lower limit; on May 27, 1998 at 2146 lb. After that date the tension readings until September 20 were above the upper limit, peaking at 6098 lb. on August 11, 1998.

The wires were ripped off from the bolt head in September. The instrumented bolt was removed on October 21 and was a newly instrumented bolt with a measured modulus of elasticity of 28,074 ksi. was re-installed on December 4, 1998.

The post with the bolt with spring washers was found dislodged on December 9, 1998. It was hit by a car and the base connection of the post broke away. Neither the bolts nor the spring washers were damaged and they were found scattered on the ground. Figures 39 and 40 show

the sign after it was hit by a car. This event, totally unexpected, provided a clear demonstration of the effectiveness of the spring washers. The post separated neatly from the base and as far as we know, no major damage was reported for the car. The sign was re-installed afterwards and since then, the readings were initially within the acceptable range. However the last readings are not reliable, because the bolt was not galvanized. The bolt deteriorated very quickly, and became completely corroded.



Figure 39 Sebastian Sign after it was hit by a car, 1st view



Figure 40 Sebastian Sign after it was hit by a car 2nd view

The bolt with flat washers was able to maintain the tension within the acceptable range until July 22. From September 11 to November 6, the readings were above 3600 lb. and the bolt started to

show erratic behavior. A test to determine if the torque and strains had a linear relationship was performed on November 11 and on November 18. We concluded that the strain gages were faulty so we took the bolt back to the lab to be re-instrumented and re-tested.

The bolt was re-installed on December 4, 1998, with a measured modulus of elasticity of 29,406 ksi. That day's reading was 3774 lb. and has been decreasing until January 29, 1999.

It was suspected that the torque wrench being used may have been inaccurate. A new torque wrench was bought and it was used to re-torque the bolt on February 5, 1999. That day's reading was high at 4300 lb. but went down to 3449 lb. on February 12. No readings were taken on February 19 because the readings were negative numbers in the thousands range. The gages were probably damaged. Subsequent readings were either below or above the allowable range. The bolt was finally removed and brought back to the lab. A tensile test showed a value of 29,276 ksi for the modulus of elasticity. Consequently, the erratic beahior of the bolt could not be attributed directly to a defect of the strain gages.

Location: Sebestian Inlet; Directly after the bridge on A1A going south.

Sign reads: "Sebastian Inlet State Recreation Area."

Description: Brown Sign - 15.5' X 5' X 9' from ground, W8X18 posts

Long Boff

															Wires ricoed off the bott	Did not propeed. Wines were summospally drawed off	Wires ricced off. Tried to recision holl, washer was shirly need help in renieving the demand hot	Wites were still finded off.	Recieced instrumented took with a receiver hort	Reciseced requier bolt with restrumented from 1 th 192 Rt. 463	Replaced witness both Retornised witness officers: 1 to 62 for \$270. New E. 94 604 v.10 fund	Rt reading changed a lot. Lt took a while to stabilize. Wines were divine the bed	Renlaced withstramented both New Officets: 1: 470 Bt. 424 New E-28 074-10 burnet	Post was dislodoed w/ bolts detached. Took hack the instrumented both	Re-Installed the instrumented bott	Re-installed & re-aligned the instrumented bott	Re-Installed the rest of the botts & re-torqued the instrumented bott	wires were dry and in the bag		rusting on all washers, bolt w/ instrumented bolt is rusted.	wires were dry and in the bag		26	320		bolt completely rusted		
Eccord	ε		0.073	0.072	0.072	0.062	0.050	0.043	0.024	0.023	0.017	0.005	910.0	0.00							0.012	-0.844	0.074		0.378	0.011	0.027	0:030	0.022	0.026	0.021	0.048	0.034	0.037	0.034	Г	Г	
Bolt tension	3		11.9	1.11	9.5	12.3	13.7	14.8	18.7	21.2	22.6	27.1	52.9	23.8							14.0	-1.3	11.8		10.7	13.5	12.7	11.4	9.3	12.1	11.8	8.5	13.5	16.5	18.8	17.1	36.3	
Bott tv	2		2671	2505	2146	2761	3075	3325	4215	4760	2086	8609	5150	5343							3148	-298	2654		2400	3039	2846	2555	2084	2716	2654	1910	3039	3706	4229	3845	8161	
(39)	Average		508	<u>8</u>	891	216	240	560	828	372	397	476	402	417							212	92	214		ğ	245	230	206	168	219	214	154	245	599	34.1	310	658	
Strain Reading (uc)	Right	194	47	45	39	7.4	111	141	246	580	326	450	338	328							\$	-200	46		973	215	164	140	128	159	167	7.5	156	180	218	160	488	
Stra	Left	479	370	346	596	357	369	378	412	463	468	505	469	206							539	160	385		-586	275	295	272	208	279	261	233	334	418	464	460	828	
Temp	ပ္	Balance Offset >>	53	34	82	33	96	31	ð	35	32	35	35	3							31	31	32		56	58	58	27	50	8	31	58	33	33	ಶ	ខ	88	
Time		Balanc		•	•	3:40	12:15	11:10	11:00	11:30	11:00	11:45	8:00	9:15						5:10	2:30	1:30	1:15	12:00	4:15	12:30	1:30	906	8:40	12:15	12:00	10:05	12:15	9:10	15:00	13:00	1:00	
# of Days	After	5/15/96	0	5	12	24	32	40	54	61	29	98	101	125	131	137	142	149	156	171	176	183	188	204	247	3 2	560	287	274	311	317	325	331	336	370	377	386	:
Date			5/15/98	5/20/98	5/27/98	96/6/9	6/17/98	96/52/9	2/8/88	7/16/98	7/22/98	8/11/98	8/26/98	9420498	9426498	10/2/98	10/7/98	10/14/98	10/21/98	11/6/98	11/11/98	11/18/98	12/4/98	12/9/98	1/22/99	1/29/99	2/2/99	2/12/99	2/19/99	3/26/99	4/2/99	4/10/99	4/16/99	4/23/99	5/25/88	6/2/86	6/11/89	_
Operator			ĭ	₹	Ŋ,	SON	NOS	NOS	NOS	NOS	SON	z	z	J.	JW.	۲,	5	Τζ	F	5	JT.	Ţ	5	۲,	5	5	5	ŀ	۲,	5	5	5	5	5	dd,	ddr	ఠ	-

Table 12 Tension and Eccentricity Values For Sebastian Inlet Sign, Bolt with Spring Washers

Location: Sebastian inlet; Directly after the bridge on A1A going south.

Sign reads: "Sebastian Inlet State Recreation Area."

Description: Brown Sign - 15.5' X 5' X 9' from ground, W8X18 posts

Short Bolt

																	Did not proposed. With suspenses transported of	Wites were in the bac.	Wires were in the bea.	Wires were in the bad.	Bec was richard off the next	Beg was ripped off the post, ITT. Re-installed same bolf & washers w/ new clears: 11: 350 An: 270	Whee were dry and in the beg. ITT 2x. Replaced instrumented both wirecular both	Recladed with instrumented both. New Offsets: Lt. 262 Rt. 821 New F=29 406×10 4 knet	Wires were dry and in the beg.		36	Re-torqued using a new torque wrench.	whee were dry and in the bad	Bolt was wet. Got blo (-) readings	Wires were dry and in the bear	PiQ	***************************************	replaced w/ regular bott permanently
Eccent	Ξ		0.048	0.057	0.058	0.059	0.058	0.058	0.056	990.0	990.0	0.063	0.050	0.055	0.059	0.058		0.062	0.061	830	0.013	0.013	0.00	890'0	0.088	0.002	9600	9000	0.002		0.184	9900	0.046	0.032
Bolt tension	ĸ		18.7	15.9	15.5	15.0	15.2	14.7	15.0	14.6	15.9	17.2	16.7	17.8	19.0	19.9		18.6	18.5	23.9	27.8	16.5	30.3	16.8	16.8	9.9	5.5	19.1	15.3		4.4	8.8	19.8	19.3
Bolt to	\$ qı		4215	3568	3491	3382	3421	3305	3376	3286	3581	3876	3747	4004	4266	4471		4183	4151	5362	6229	3703	6822	3774	3780	1494	1247	4300	3449		966	1988	4443	4346
(an)	Average		329.0	278.5	272.5	264.0	267.0	258.0	263.5	256.5	279.5	302.5	292.5	312.5	333.0	349.0		326.5	324.0	418.5	488.5	289.0	532.5	290.5	291.0	115.0	0.96	331.0	265.5		76.5	153.0	342.0	334.5
Strain Reading (uc)	Right	821	160	109	104	98	103	96	106	102	114	132	135	129	124	133		Ξ	112	287	421	248	539	200	565	112	59	360	560		7	45	209	450
Strai	Left	282	488	448	441	430	431	418	451	411	445	473	450	967	542	565		542	536	220	556	330	226	8	17	118	133	305	172		221	285	175	219
Тетр	ç	Balance Offset >>	53	34	28	33	36	31	8	છ	æ	8	8	31	31	41		43	96	59	20	33	છ	8	35	26	31	82	27	20	58	32	8	33
Time		Balanc			•	3:40	12:15	11:10	11:00	11:30	11:00	11:45	8:00	9:15	8:35	12:55		3:00	1.25	3:50	4:50	2:50	2:50	5:00	12:10	4:25	12:50	1:45	9:10	8:45	12:20	12:05	10:10	12:20
# of Days	After	5/15/9 6	0	5	12	24	æ	40	75	91	67	88	101	125	131	131	137	142	149	156	171	178	183	- 66	Š	247	224	260	267	274	311	317	325	331
Date			5/15/96	5/20/98	5/27/98	96/6/9	6/17/98	6/25/98	96/6/2	2/16/98	7/22/98	8/11/96	8/26/98	9/20/98	9/26/98	9/26/98	10/2/98	10/7/98	10/14/98	10/21/98	11/6/98	11/11/98	11/18/98	12/4/98	12/9/98	1/22/99	1/29/99	2/5/99	2/12/99	2/19/99	3/26/39	4/2/99	4/10/99	4/16/99
Operator			ĭ	¥	¥	SON	SON	SON	SON	SON	NOS	z	z	¥	¥	¥	75	5	5	Τſ	5	5	5	۲,	Į,	۲,	5	5	5	Ę	5	5	5	٦٢

Table 13 Tension and Eccentricity Values For Sebastian Inlet Sign, Bolt with Flat Washers

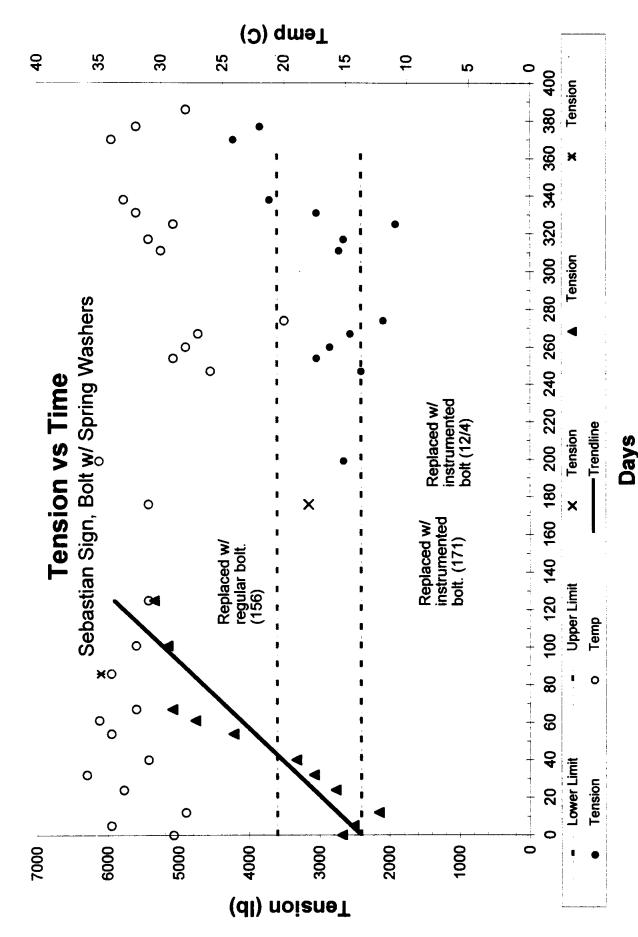


Figure 41 Tension vs Time, Sebastian Inlet Sign, Bolt with Spring Washers

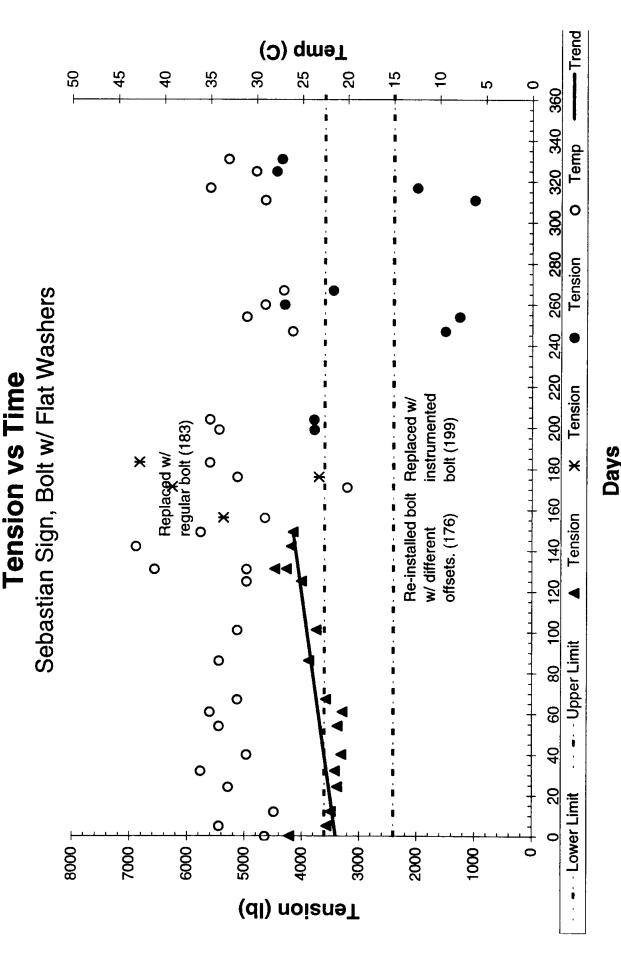


Figure 42 Tension vs Time, Sebastian Inlet Sign, Bolt with Flat Washers

Indian River

The Indian River sign bolts were installed on May 15, 1998 and they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located southbound along I-95 just south of the 146-mile marker. It is bolted down using 3/4" diameter bolts. Figure 43 shows the location of the sign with respect to I-95.

Tables 14 and 15 list the results of monitoring the bolt from May 15, 1998 until July 6, 1999. Figures 44 and 45 show a plot of the tension vs. time for the bolt with the spring washers and the bolt with flat washers.

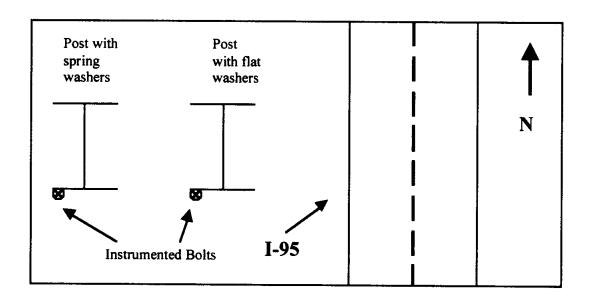


Figure 43 Location of Indian River Sign with respect to I-95

For the Indian River bolt with spring washers, the readings from May 15, 1998 through June 9, 1998 were all under the lower limit of 2400 lb. The bolt was then re-installed and it maintained the tension within the acceptable range (2400 lb. to 3600 lb.) from June 1998 through early December 1998. From June 17, 1998 through January 18, 1999 the left strain gage read a negative strain indicating that the bolt was subjected to a combination of tension and bending. This is reflected in the big eccentricities in the readings.

The initial readings taken during January 1999 showed that the tension increased, so we brought the bolt back to have the strain gages replaced. After replacing the strain gages, the spring washers were able to maintain the tension within the acceptable range until this date.

The Indian River bolt with flat washers was not able to maintain tension within the acceptable range. The eccentricities from May 15, 1998 through February 19, 1999 were small which meant that the bolt was not subjected to any significant bending.

This bolt has been re-installed four times. It was removed on June 17 because that day's readings were too high. The bolt was re-installed in June 25, 1998 and the readings had been below 2400 lb. up to August 26. After that date, the readings were well above the upper limit. It was replaced on October 21 and a new one was re-installed on November 6. The reading on that day was high while the following reading on November 11 was low. The bolt was re-instrumented and re-installed on December 9 with a modulus of elasticity of 28,706 ksi. The readings after that date were low until January 18, 1999 when the wires were ripped off. A newly instrumented bolt was re-installed on February 5, 1999 with a measured modulus of elasticity of 28,120 ksi. The readings since then have been low up to July 6, 1999.

Location: Indian River, I-95 South of Vero Beach (mile post 146)

Sign reads: "Indrio Rd. 8, Miami 146."

Description: Green Sign - 14' X 5.5' X 10' from ground, WBX24 posts

																														and a	Re-installed new instrumented bott. Lt. (-21/858 Rt: (-21/700, New E=28.541 x 106kps)										
Eccent.	(in)		0.036	0.039	0.037	0.035		0.182	0.204	0.204	0.202	0.220	0.214	0.195	0.207	0.197	0.201 Left & right labels were removed	Υ.			_	_	0.1/8 with the bag only and in the bag	_	Ť		1	U.124 Wires were dry and in the bag	0.124	Reclaced instrumented bott with regular host	_	•	7	039 nusting on all washers	_	_		•	т.	8000	
	KN		0.0	9.0	7.9	8.1		15.3 0.	11.9	11.9 0.2	11.9 0.	10.9	11.4 0.	12.2 0.	12.5	T	12.5 0.	П	Н	+	+	+	9 6	t	T	14.6 0.	7	0 0	†	t	14.9	12.8 0.0	┪	┢	H		H	12.3 0.0	T	t	
Bolt tension	lbs	H	2018	1787	1768	1813		3434	2684	2665	2684	2460	2556	2735	┝	2883	2799	L	-	+	+	+	3336	╀	L	Н	\dashv	2000	╀	L	_	-	H	L	2736	_	L	_	_		
(g)	Average	Ц	158	140	138	_		L	210	508	_	192	500	214	219	225	L	233	\dashv	240	+	+	+	229	H	Н	+	2 5	+	H	H	-	228	-	Н	H	H	L	212	L	
Strain Reading (ue)	$\overline{}$	(-2K)700	26	88	3 5	88	bott removed, tested and retorqued	788	999	099	199	642	655	657	702	969	889	692	969	713	732	3 2	96/2	757	752	757	759	404	878		134	138	121	122	130	134	118	140	121	122	2
Strair	-	(-2k)858	218	197	192	2 8	wed, tested a	-252	-247	-244	-242	-558	-256	-530	7	-248	-25	-227	-235	-233	-547	į į	5 6	8	-245	-246	-252	\$ 02.	8 28		386	317	334	298	304	306	252	862	305	200	101
Temp	ပ္	Balance Offset >>	58	58	27	ş	boft remo	35	28	32	88	52	33	8	42	41	8	37	31	8	R	8 8	٩٤	2	23	5 8	22	8 8	2 2		88	8	58	30	35	98	28	37	31	æ	2
Time		Batance				12:40		10:00	9:00	9:00	9:45	00:6	9:30	8:45	9:30	1:45	3:05	10:50	10:00	10:20	1:55	10:30	200	88	9:00	11:00	12:00	20.6	530	8:50	9:05	11:45	10:35	9:02	9:08	11:45	8:55	11:35	11:30	Ę	}
# of Days	After	5/15/96	0	5	12	7,		æ	40	54	61	29	88	101	131	131	137	142	149	156	121	9/1	3 2	ž.	209	508	8	243	243	254	260	267	274	311	317	325	331	338	37.7	305	3
Date			5/15/98	5/20/98	5/27/98	96/6/9		6/17/98	6/25/98	2/9/36	7/16/98	7/22/98	9/11/96	8/26/98	9/26/98	9726/98	10/2/98	10/7/98	10/14/98	10/21/98	98/9/1	11/11/86	12/4/00	12/9/98	12/14/98		30000	1/10/03		1/29/99	2/5/99	2/12/99	2/19/99	3/26/99	4/2/99	4/10/99	4/16/99	4/23/89	6/2/86	6/11/00	8
Operator			Ä	ML	ML	NOS		NOS	NOS	SON	NOS.	SON	z	z	¥	ML			7	5!	Ť	5 5	t	5	Τſ		ļ	5		15	ΤÇ	11	Τſ	T,	Τſ	ΤĊ	ΤĊ	JT.	ddf	2	5

Table 14 Tension and Eccentricity Values For Indian River Sign, Bolt with Spring Washers

Location: Indian River, I-95 South of Vero Beach (mile post 146)

Sign reads: "Indrio Rd. 8, Miemi 146."

Description: Green Sign - 14' X 5.5' X 10' from ground, WBX24 posts

Short Bolt

							bott removed and re-instrumented (bott very sticky, a lot of friction when retorns ed)										Wires were in the hear.	Wires were in the bed	Recladed Instrumented bolt with a marker both	Replaced recialar bolt with an instrumented but 11: 78 Bt (490)	Tension was low. Torqued again and average staved markly the came	Recladed instrumented both with short both	Have not replaced the requise both with an instrumented half were	Recladed regular bolt with an instrumented hot. It: 8 Pt (-24)845 New F=29 708*1064ms/	Wires were in the bad.		949	Wires were ripped off. Replaced with regular bolt.	Was not able to re-install infrumented bott. After testing, E=25.796 (too low)	Reinstalled inst. bolt winew wrench. Lt. (-2k)796 Rt. (-2k)477, New E±28,121x tostroe!	wires were dry and in the bag	92	2	28	28	: 28	2				
Eccent	Ē		0.027	0.070	0.049	9200	990.0	0.013	0.009	9000	0.004	0.002	0.003	0.019	0.017	0.019	0.018	_		0.083	1	_		9000	т	•	0.024			0.084	0.085	0.087	Т	1	T	T	1	T-	0.072	0.177	
Bolt tension	ž		10.0	25.8	13.7	5.5	23.4	7.7	8.3	8.3	9.1	9.0	9.0	40.7	40.7	41.3	40.2	39.4		28.7	5.5			4.3	3.0	3.4	3.7			9.8	9.4	4.6	8.2	9.3	9.1	7.7	9.0	9.7	8.3	4.9	
Bolt	ĝ		2255	5810	3068	1236	5253	1672	1877	1871	2043	2031	2031	9141	9148	8282	9045	8853		6455	1239			970	672	774	831			2199	2124	1888	1851	2087	2037	1739	2025	2174	1863	1106	
(E)	Average		176	454	241	97	410	131	147	146	160	159	159	714	714	725	902	691		412	62			1	53	61	99			177	171	152	149	168	164	140	163	175	35	28	
Strain Reading (uc)	Right	(-2k)477	126	117	116	123	120	112	160	159	152	155	154	268	285	575	267	552		774	အ			02	44	52	49			336	326	293	284	589	307	560	598	300	265	257	
Strail	Left	(-2k)796	526	790	996	۶	200	149	133	133	167	162	163	628	846	874	945	830		49	95			83	62	۶	88			18	16	11	14	47	21	82	82	S	38	62.	
Temp	၁	Offset >>	58	28	22	Q	38	58	35	æ	22	33	8	42	41	34	37	90		30	34			8	82	×	23			27	8	28	31	33	38	96	37	88	28	8	
Time		Balance	•	•		12:40	10:00 35	00:6	9:00	9:45	9:00	9:30	8:45	9:30	1:45	3:05	10:38	10:10	10:30	2:10	10:45	9:30	9:15	9:00	9:10	11:00	12:00	9:55	R	9:25	11:55	10:40	9:10	9:10	11:50	9:00	11:40	11:30	1:00	7:15	
# of Days	After	5/15/98	0	2	12	24	35	40	35	61	67	88	101	131	131	137	142	149	156	171	176	183	2	204	508	508	ĝ	243	Š	280	267	274	311	317	325	331	338	377	386	411	
Date			5/15/98	5/20/98	5/27/98	96/6/9	6/17/98	6/25/98	7/9/96	2/16/98	7/22/98	8/11/96	8/26/98	9/26/98	9/26/98	10/2/96	10/7/98	10/14/98	10/21/98	11/6/98	11/11/98	11/18/98	12/4/98	12/9/98	12/14/98			1/18/99	1/29/98	25/89	2/12/99	2/19/99	3/26/99	4/2/99	4/10/99	4/16/99	4/23/99	66/2/9	6/11/9	7/6/99	
Operator			₹	¥	¥	SON	SON	SON	SON	NOS	SON	z	z	¥	¥	Ļ	5	Τſ	5	5	5	5	5	5	5			5!	5	5	5	5	5	۲,	Ŋ	5	5	<u>&</u>	ddf	dd,	JPP

Table 15 Tension and Eccentricity Values For Indian River Sign, Bolt with Flat Washers

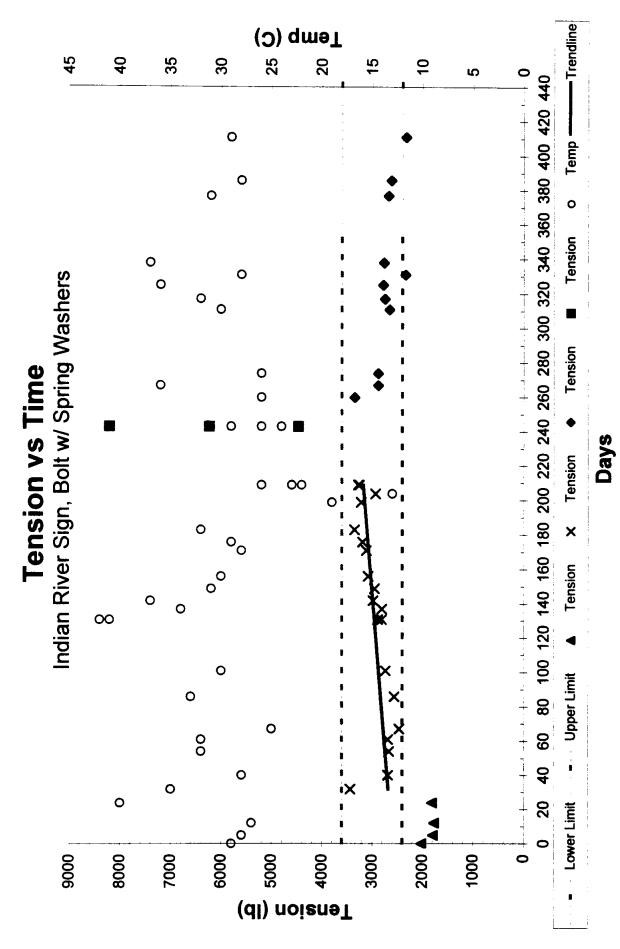


Figure 44 Tension vs Time, Indian River Sign, Bolt with Spring Washers

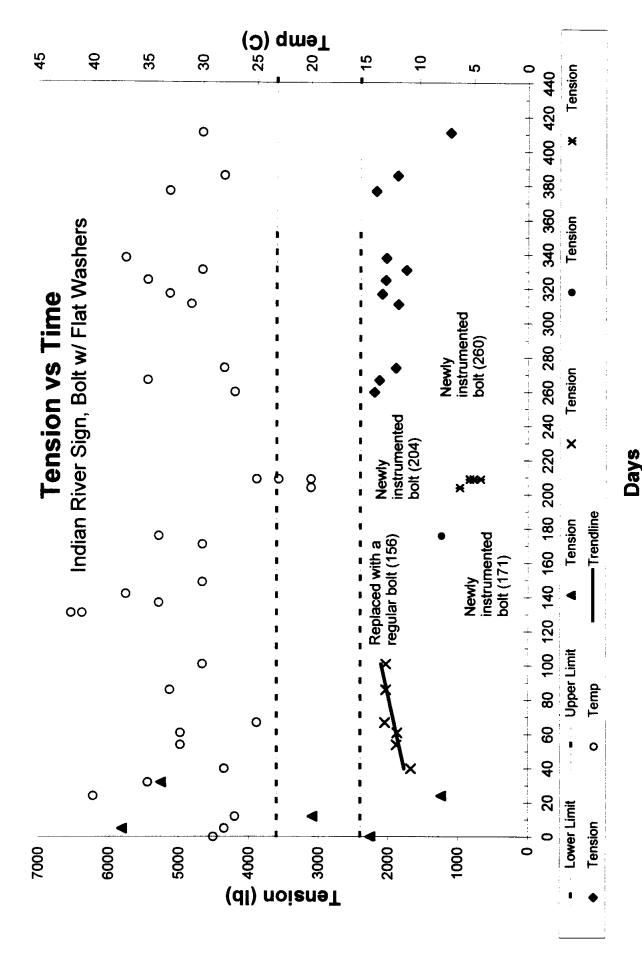


Figure 45 Tension vs Time, Indian River Sign, Bolt with Flat Washers

Martin County

The Martin County sign bolts were installed on May 15, 1998 and they have been monitored ever since. Different students have been taking readings ever since they were installed. The sign is located northbound along I-95 just south of the Exit 62. It is bolted down using 3/4" diameter bolts. Figure 46 shows the location of the sign with respect to I-95.

Tables 16 and 17 list the results of monitoring the bolts from May 15, 1998 until April 16, 1999. Figures 47 and 48 show a plot of the tension vs. time for the bolt with the spring washers and the bolt with flat washers.

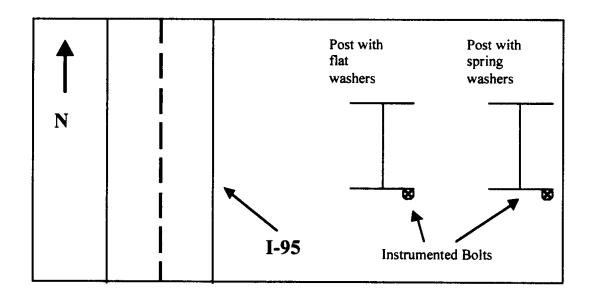


Figure 46 Location of Martin County Sign with respect to I-95

The Martin County bolt with spring washers was able to maintain the tension close to the acceptable range until August 8. It was submerged under water for at least a day on August 11 after which on August 26, the tension reading was 3786 lb. It was again submerged under water on September 10, 1998. From October 2 to October 21, 1998, the tension readings ranged from 4638 lb. to 4894 lb. Again, the bolt was submerged in water on November 6, 1998.

On November 11, 1998, a test was performed to determine if the torque and the strains had a linear relationship and check if the strain gages were working properly. Another test was performed on November 18 and the torque and strain registered a non-linear relationship. Thus we concluded from the tests that the bolt had to be replaced. The bolt was taken back to the lab to be re-instrumented and re-tested.

The newly instrumented bolt was re-installed on December 4, 1998 with a measured modulus of elasticity of 28,664 ksi. Since then the tension readings were out of the acceptable range on four occasions, two of which were over the 5000 lb. Mark. On April 16, the reading was abnormally high, and the bolt was removed. A tensile test in the lab yielded a value of 30,054 ksi for the modulus of elasticity, a 5% increase over the initial value measured before installation in the field.

The bolt with flat washers was able to maintain the tension within the acceptable range until August 26 even though the bolt was submerged in water on August 10 and 11. The bolts were again submerged in water on September 7. In between those dates, a reading was attempted; however, the right gage read an infinite resistance, which prevented a reading to be taken. The readings fluctuated from 1826 lb. on October 14 to 4683 lb. on October 21.

The bolts were again submerged in water on November 6 and 11. On November 18, the right gage was shorted out and so the bolt was taken back to the lab. The bolt was re-installed on December 4, 1998 with a measured modulus of elasticity of 29,687 ksi. Since then, the readings fluctuated with a downward trend, until the bolt was removed on April 10, 1999. A tensile test in the lab yielded a value of 31,912 ksi for the modulus of elasticity, a 9% increase over the initial value measured before installation in the field.

Location: Martin; After mile post 109 going north.

Sign reads "Exit 62, 714,714,1/2 mile."

Description Green Sign - 10' X 10' X 12' from ground, W8X24 posts

													Sign Boths under water		Sign Bolts under water	Bots unaccessable due to muddy conditions	Wires were in the bad	Wires were in the beat. Was not able to take temperature	off the thermocouple at the Indian River site	Wires were in the bad	Wires were in the bag	Botts were totality submerged	IT. Re-installed same bolt & washer with new offsets: Lt: 236 Rt: 114	ITT. Non-linear results. Replaced instrumented bott with regular one.	Replaced with instrumented bott. New Offsets: Lt. 210 Rt. 118.	New E = 28.664x10 hpsi	Wires were in the bag		Wires were in the bag					Rusting on all washers			replaced w/ regular bolt permanently
	Eccent.	(ju)		0.011	0.027	00:00	0.030	0.031	9.034	0.034	0.035	0.034	3	0.033	3	<u>*</u>	960.0	1	Т	<u>₹</u>	_	ĕ	0.066	_	0.002	_	0.017 W	0.014 Ibid	_	_	0.020 Ibid	_	0.024 thi	790.0	_	7	0.035
Ì		KN		13.6	15.5	15.5	17.1	9.71	16.8	17.6	17.71	16.5		16.8			20.8	50.6		Г	21.8		15.7		11.9	1	18.3	1	┪	15.6		٦	Т	13.4	П	Н	28.3
	Bolt tension	lbs		3068	3491	3485	3850	3952	3786	3952	3972	3709		3786			4676	4638		4644	4894		3830		2678		4109	3432	3660	3501	5154	3406	5787	3014	3368	3229	6357
	E .	Average		240	273	272	<u>8</u>	306	982	906	310	290		58g			365	362		363	382		276		212		325	271	88.	277	407	569	457	238	566	255	205
	핡	Flight A	118	211	±35	186	205	505	-88	198	193	186		192			223	218		220	234		470		98	ŀ	382	312	200	96	321	169	341	[29	73	88	313
	Strain	Left	210	568	350	358	986	412	4 02	419	427	393		386		:	507	206	-	202	530		- 81		217		764	230	3/2	<u>8</u>	493	88	573	609	459	452	169
}	Temp	ပူ	Offset >>	35	8	27	35	37	8	38	ਲ	30		8			8			35	38		35	8	22	ļ	8	7	5	31	33	8	31	30	ಜ	æ	8
	e E		Balance Offset >>					11:00	10:00	10:00	10:30	9:50		9:30	1:29	7:34	4:45	12:20	_	11:25	11:25		12:00	10:40	10:15		10:10	01:01	00:1	10:20	10:45	12:50	11:35	10:10	10:00	12:40	10:05
-	# Of Days	After	5/15/98	0	5	12	24	35	0*	54	19	67	98	101	115	115	137	142		149	156	171	176	183	199		Ř	500	243	254	98 98	267	274	311	317	325	331
	Cate			5/15/98	5/20/98	5/27/98	86/6/9	6/17/98	6/25/98	2/9/38	7/16/98	1/22/98	8/11/88	8/26/98	9/10/98	9/10/98	10/2/98	10/7/98		10/14/98	10/21/96	11/8/98	11/11/98	11/18/98	12/4/98		12/9/96	12/14/98	1/16/99	1/29/89	2/5/99	2/12/89	2/19/99	3/26/99	4/2/86	4/10/99	4/16/99
Long Bolt	Operator			ML	ML	ML	SON	SON	SON	SON	SON	SON	Z	Z	¥	Mf.	Τſ	Τſ		7	5	1	5	1	┥	ŀ	1	5	†	+	5	5	5	5	5	5	F 5

Table 16 Tension and Eccentricity Values For Martin County Sign, Bolt with Spring Washers

Location: Martin; After mile post 109 going north.

Sign reads "Exit 62, 714,714,1/2 mile."

Description Green Sign - 10" X 10" X 12" from ground, WBX24 posts

Short Bolt

												Stan Bolts under water		Ston Botts under weter	Bolts inaccessible due to murbly conditions	Right mane was notatiful aborted out. It ment an infinite maintena		Wires were in divisid in the bad. Blott date read a resistance of 25 ohms.	Wines were in dry and in the bag. Out and spliced both wines	Right gage initially 439 then It fluctuated.	Bolts were totally submerged.	Botts were totally submerced.	Wires were wet. Rt Gade was shorted out. Renlaced with necessar both	Replaced with instrumented both, New Offsets: Lt. 298 Rt. 208	New E = 29.687x10 kms	wires were dry and in the bag	PiQ					PiQ	900	999	-0.316 replaced with regular bott permanently
Eccent.	Ē	L	900.0	900.0	0.007	0.010	0.012	0.012	0.014	0.013	0.016		0.013					960.0	7					0.046	_	0.075	090.0	1	0.062	0.073	8	160.0	т	0.349	-0.316
Bolt tension	ĸ		17.0	14.5	14.2	14.6	14.2	13.8	13.6	1.4	13.9		14.6					8.1	20.8					15.5		21.0	16.4	9.2	6.2	7.6	5.1	17.5	7.	9.0	-1.3
Bolt to	sq _i		3818	3267	3184	3293	3203	3113	3068	3171	3120		3280				mged.	1826	4683					3476		4722	3679	2059	1390	1705	1141	3928	306	144	-282
(me)	Average		288	255	249	257	250	243	240	248	244		256]			Botts were inaccessible, they were totally submerged.	143	396					265		360	281	157	106	130	87	300	24	11	-22
Strain Reading (uc)	Right	208	316	270	268	284	283	274	275	282	282		292				they were	0	439					394		648	09*	218	176	231	132	290	*	-30	ģ
Stra	Left	296	280	240	523	230	217	212	200	213	202		220			282	neccessible	285	282					136		72	101	96	36	28	42	6	43	52	51
Temp	ပ္	Balance Offset >>	35	90	22	ક્ષ	37	34	ક્ક	ક	8		œ				Bofts were it	33	8					29		34	23	31	30	32	38	31	32	98	36
Time		Balance	,		•	5:00	11:00	10:00	10:00	10:30	9:50		9:30	1:29	7:34	4:55	12:30	11:35	11:30			11:55	11:10	10:45		10:15	10:15	11:05	10:25	10:55	12:55	11:40	10:15	10:05	12:45
# of Days	After	5/15/96	0	2	12	24	32	40	54	61	- 82	98	101	115	115	137	142	149	156		171	176	183	199		504	508	243	254	5 80	267	274	311	317	322
Date			5/15/98	5/20/98	5/27/98	96/6/9	6/17/98	6/25/98	96/6/2	7/16/98	7/22/98	8/11/88	8/26/98	9/10/98	9/10/98	10/2/98	10/2/98	10/14/98	10/21/98		11/06/98	11/11/98	11/18/98	12/4/98		12/9/98	12/14/98	1/18/99	1/29/89	2/5/89	2/12/99	2/19/99	3/26/99	4/2/99	4/10/99
Operator			¥	¥	ML	NOS	NOS	SON	NOS	NOS	NOS	z	Z	ML	ML	J,	ΤĽ	F	5		5	5	5	Į,		5	5	Ţ	5	5	5	5	5	ĴΤ	5

Table 17 Tension and Eccentricity Values For Martin County Sign, Bolt with Flat Washers

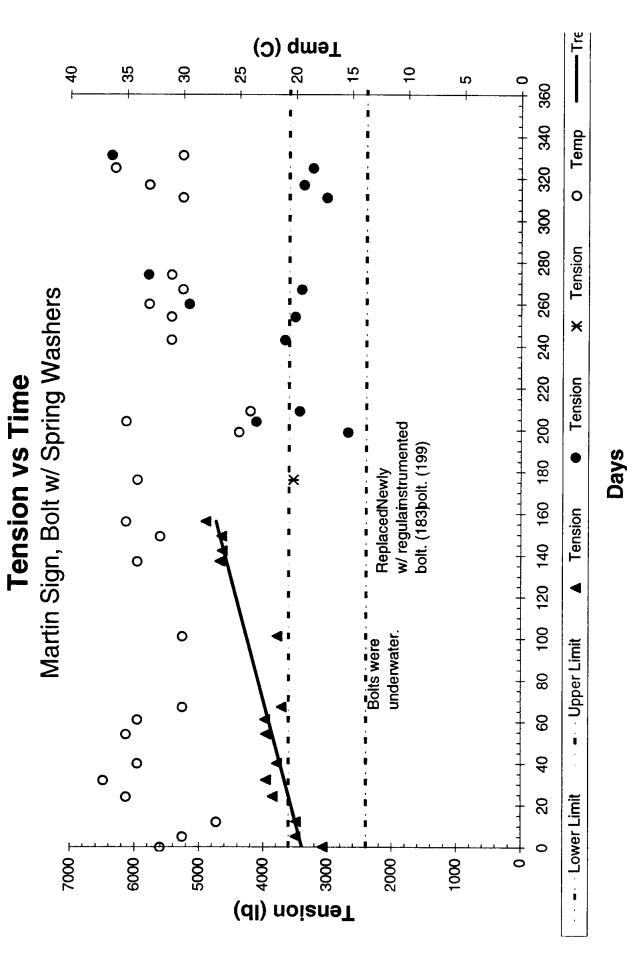


Figure 47 Tension vs Time, Martin County Sign, Bolt with Spring Washers

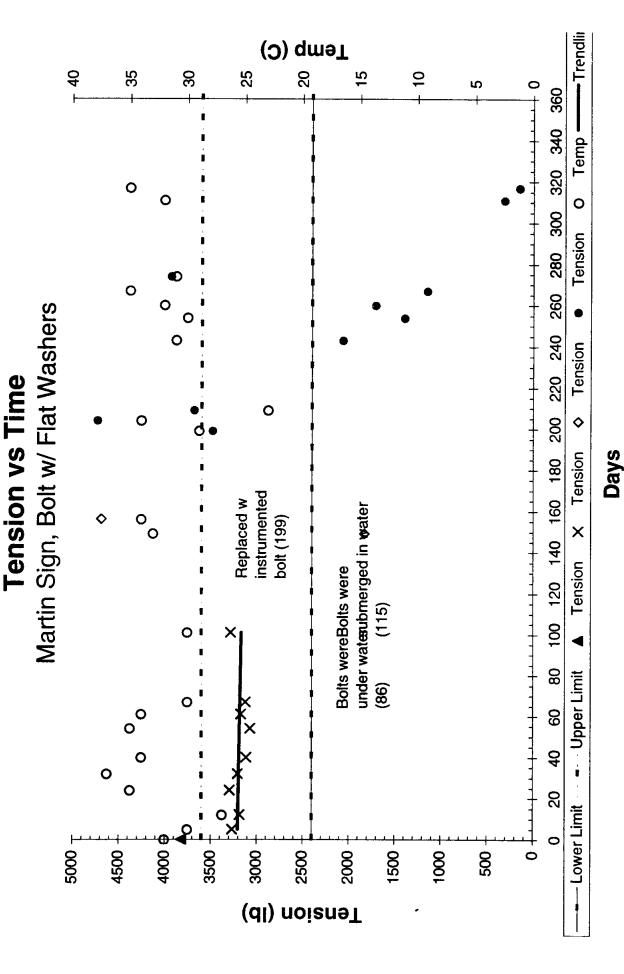


Figure 48 Tension vs Time, Martin County Sign, Bolt with Flat Washers

TEMPERATURE TESTS

In some cases it appeared that there was some correlation between the tension readings and the temperature readings. The tension in the bolt goes up or down with the temperature. In particular, this happened for the Sebastian sign bolts both with spring washers (Figures 41); for the First Brevard sign bolts both with and without spring washers (Figures 36 and 37); for the 2nd Brevard sign bolts both with and without spring washers (Figures 33 and 34); and, for the Orlando sign bolt with spring washers (Figure 30). It must be noted, though, that the temperature readings, measured on the surface of the bolt, were indicative of the ambient temperature, and that the temperature of the bolt core was impossible to record in the field.

There were some concerns that the strain gages or the adhesive between the gage and the bolt could be affected by the temperature variations despite the manufacturer assurances to the contrary. Consequently, temperature tests were performed to determine the temperature effects, if any, on the strain gage readings. Several temperature tests were performed on bolts when they were returned from the field.

The procedure for the temperature tests was as follows:

- The untensioned bolts were placed into an Equatherm Oven manufactured by Curtin
 Matheson Scientific Inc. The bolts were connected to a P-3500 strain indicator to record the
 strain as the temperature was increased. An Acu-rite oven thermometer measured the
 temperature of the bolt.
- The temperature was increased from an initial ambient bolt temperature of around 24 °C to a maximum temperature of above 80 °C. The strains were measured for temperature increments of around 10 °C. The strain readings were also taken at similar decrements back to the ambient temperature of the bolt.

The results for the tests performed on the Orlando long bolt are shown below. The bolt was tested in three different settings. Figure 49 shows the result of the temperature test for the stand alone bolt, untensioned. Figure 50 shows the result of the temperature test for the bolt installed and tensioned on an aluminum fixture with spring washers. Figure 51 shows the result of the temperature test for the bolt installed and tensioned on an aluminum fixture with flat washers. In

all the tests, it can be seen that the variation in strain is small, consistent with the expected thermal expansion of the steel bolt for this range of temperature. It was then concluded that the strain gages were not adversely affected by the expected temperature changes, and that the bolts could be used as intended in the field.

However, the results of the test do show that the slope of the strain-temperature curve varied for each test. This is to be expected, since the aluminum fixture, the galvanized steel flat washers, the spring washers, and the bolt itself are made of different alloys with different coefficient of thermal expansions. The interactions between the different elements of the connection result therefore in a different rate of thermal expansion or contraction, which may explain the variation of tension with the temperature.

Consequently, the observed variation in tension with the temperature cannot be attributed to the strain gages. The probable explanation is that the sign structures themselves expand or contract non uniformly when subjected to the action of the sun. One face of the posts might be warmer than the other, or one post might be in the shadows of trees while the other is fully exposed to the sun. The differential expansion and/or contraction of the sign might induce additional forces on the break-away connections. These in turn will result in changes in tension of the bolts.

These changes were also documented over a one day period by taking measurements of the same bolt at different times the same day. See readings for days 138 and 147 of the First Brevard sign in Tables 10 and 11; for days 196 and 205 of the 2nd Brevard sign in Tables 8 and 9; for days 196 and 205 of the Orlando sign in Table 6; for days 209 and 243 of the Indian River sign in Tables 14 and 15. Depending on the sign sun exposure and location of the bolt, the changes in tension during the day can be large or small. That will also explain in part the better performance of the spring washers. By design, the spring washers can accommodate these changes in tension and bring back the tension in the allowable range. On the contrary, after several of these temperature cycles, the bolts with flat washers might tend to permanently loosen.

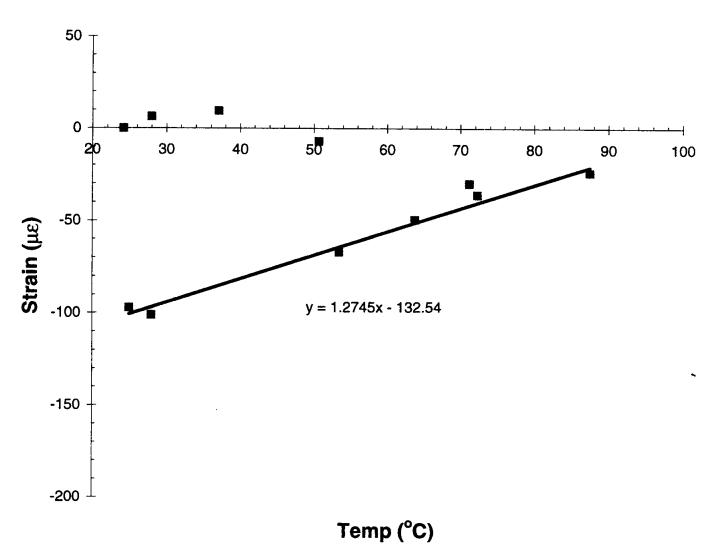


Figure 49: Strain vs Temperature, Orlando Sign, Long Bolt Untensioned, 5/13/99

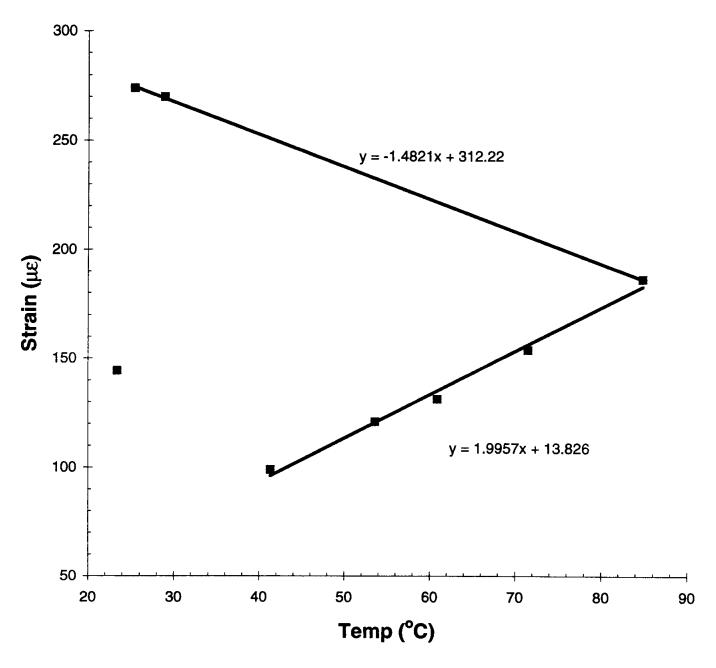


Figure 50: Strain vs Temperature
Orlando Sign, Bolt w/ Spring Washers, Tensioned, 5/6/99

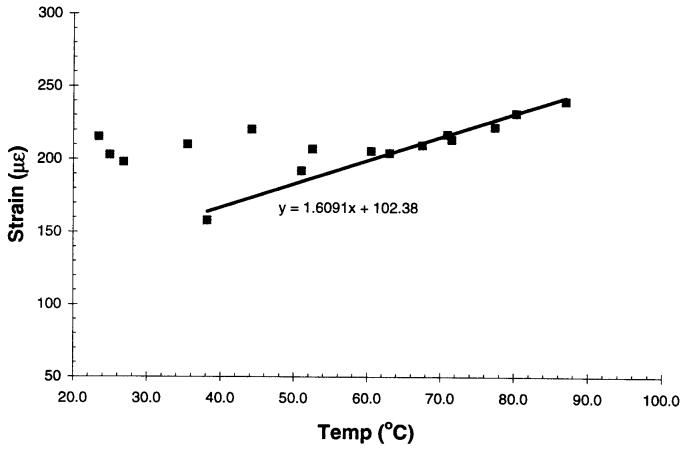


Figure 51: Strain vs Temperature
Orlando Sign, Long Bolt w/ Flat Washers, Tensioned, 5/7/99

DISCUSSION

Tension in the bolts of seven highway signs were monitored over a period of a year, from May 1998 to the present. Seven bolts were equipped with conventional flat washers, while the other seven were installed with Belleville spring washers. However, it was impossible to achieve a continuous record of measurements for each of the 14 bolts monitored. Most of the strain gages had to be replaced at one point or another after being damaged accidentally by the weather, flooding, grass mowers, or other causes. Therefore it is somewhat difficult to show definite long term trends in the behavior of the bolts. However, there were clear differences between the two sets of specimens. They are summarized below.

Bolts with Spring Washers

Long term behavior

The bolts equipped with spring washers appeared to have a better, more predictable behavior. They were able to maintain the tensions within the acceptable range for five out of seven signs.

The Orlando sign is the only sign that was never damaged, and therefore was never re-torqued. It is the only sign for which we have a single continuous record of measurements. Throughout the initial 10 month monitoring period, tension readings were all within the acceptable range. Only at the end of the monitoring, during the spring of 1999, the readings went below the allowable range, probably because of a defective gage.

The Indian River and Brevard (2) bolts with spring washers were able to hold the tension within the acceptable range for approximately 200 days after they were torqued. Lamentably, both bolts had to be re-installed in February 1999. Since then, the tension readings kept within the acceptable range.

The Tampa bolt with spring washers was able to hold the tension within the acceptable range for 112 days after it was first installed. Lamentably, the left wire was cut on October 1998 and the bolt was not re-installed until February 1, 1999. On February 8, 1999, the initial readings were too low so the bolt was re-torqued using a new spring washer stack. After an initial peak, the tension readings have kept within the acceptable range to date.

The Brevard (1) bolt with spring washers were able to hold the tensions in the acceptable range for approximately \$5 days, after the initial installation. However, the bolt had to be re-installed on November 20, 1998 and the tension readings were initially slightly above the upper limit of 3600 lb and then stayed in the allowable range. The temperature readings for this particular series seemed to follow the trend of the bolt tensions.

The Martin and Sebastian bolts with spring washers had several problems with their strain gages throughout the testing program, so no definite trend could be observed.

The results of the monitoring of the bolts showed that the spring washers can maintain the tension in the bolts for extended periods of time. The Orlando sign is a prime example. The bolt has never been re-torqued since it was installed on May 16, 1998 and yet, the tension was maintained within the acceptable range for most of the experiment. It should be noted that the Orlando sign is in a high-traffic area and this fact supports the ability of the spring washers to maintain tensions more efficiently than regular flat washers do.

The Indian River results also show that the spring washers can maintain the tension in a bolt even if the bolt is not properly aligned. From June 17, 1998 to January 18, 1999, the left gage registered negative strains, as a result of a big eccentricity in the bolt. Nevertheless, the spring washers were able to maintain the tension for a majority of the time.

Finally, and very importantly, the connection with spring washers displayed outstanding behavior during a car accident, as described above in the section on the results of the Sebastian Inlet bolt.

Galvanized vs. stainless washers

At each sign location, three bolts were assigned galvanized washers, and one bolt was assigned stainless washers. To date, no significant differences were observed between the two types of washers, in term of behavior, degradation and rust. Rust was observed in both cases. However it seems to be more related to the fact that the washers come prepackaged in a plastic wrap that traps the humidity. In all cases, the rust appeared on the edges of the washers.

Installation procedure

The tension in the bolts depends on the deflection of the spring washers. Consequently, the investigators have proposed an installation procedure based on measuring the deflection with filler gages. The procedure is described with details in the Phase I report [1]. Over the course of the study, a total of 21 installations of instrumented bolts with spring washers were performed with this method. Table 18 summarizes the results of these installations. For each of the seven signs, the different installations are listed, with the corresponding dates and the values of the tension measurements for the two readings following each installation. We are including the second reading in the table, because we observed that sometimes, although the first readings indicated a value outside of the allowable tension range, after a short initial period of adjustment, the second and subsequent readings fell between the range.

It can be seen that out of 21 cases, 81% (or 17 cases) were successful installations, and only 19% (or 4 cases) were unsuccessful. Successful means that the tension in the bolt was in the allowable range. This is a fairly high rate of success, specially compared to the 32% rate of success for the traditional torquing method, as described in the next section. However, we feel that the method could be improved upon by designing a special instrument to measure the deflections. Also, it was observed that lubrication of the connection prior to installation improves the chances of success.

	Reading #	1	2
Sign	Date	(lb.)	(lb.)
Tampa	5/18/98	2178	1806
·	6/10/98	1877	1883
	2/1/99	2755	
	2/8/99	4093	3024
Orlando	5/16/98	3382	3651
Brevard (2)	5/16/98	2787	3299
.,	2/12/99	3333	2960
Brevard (1)	7/14/98	3697	2956
• •	11/20/98	3880	3836
Sebastian Inlet	5/15/98	2671	2505
	11/11/98	3148	-298
	12/4/98	2654	
	1/22/99	2400	
	1/29/99	3039	
	2/5/99	2846	2555
Indian River	5/15/98	2018	1787
	6/17/98	3434	2684
	2/5/99	3341	2869
Martin County	5/15/98	3068	3491
•	11/11/98	3530	2.2.
	12/4/98	2678	4109

Table 18: Tension in Bolts with Spring Washers

Maintenance

The maintenance procedure should be the same than for the traditional flat washer connections. It is emphasized that a better installation and latter performance will be achieved if the bolt and washers are initially lubricated. In addition, special care should be given to the filler gages used for installation. They should be conveniently oiled and kept without corrosion, to ensure proper measurement of the spring washer deflections. The bolts and stacks of washers should be replaced periodically in accordance with manufacturer prescriptions, and FDOT maintenance schedules [1].

Bolt with Flat Washers

Long term behavior

The tension readings in the bolts with flat washers generally were not within the acceptable range. In four out of seven cases, the tensions in the bolt decreased over time. In four out of seven cases, in spite of being installed with the proper torque according to FDOT specifications, the bolts remained under-tensioned. In one case the bolt was over-tensioned despite being torqued to the proper FDOT specifications.

The Orlando sign registered tensions below the lower bound with a decreasing trend since it was first installed in May 16,1998. The bolt has been re-torqued six times and four out of these five times, the bolt was under-tensioned. Despite using a new torque wrench, two out of the three times it was used, the bolt still remained under-tensioned. The left gage has almost always registered a negative strain since June 10, 1998. This resulted in a big eccentricity in the bolt. One possible cause for this result is that the different connection components are not properly aligned.

The Indian River bolt had only one tension reading within the acceptable range. In spite of being torqued to the proper torques according to FDOT specifications, the bolt still remained undertensioned.

The Brevard (2) bolt also had only a few tension readings within the acceptable range. In spite of being torqued to the proper torques according to FDOT specifications, the bolt still remained under-tensioned.

The Tampa sign registered values in the allowable range with peaks above the range, and the tension readings fluctuated a lot.

The Martin sign was able to maintain the tension within the range for 101 days. After it was replaced on December 4, 1998, the tension fluctuated severely.

The Sebastian sign registered an increasing trend. The bolt was re-installed on November 11, 1998 and the tension readings were above the upper bound of 3600 lb. The bolt was again re-

installed on December 4, 1998 and since then, the tension has only been in the acceptable range once.

The Brevard (1) sign has never been re-torqued since it was installed in July 14, 1998. The results have always been below the allowable values.

The results of monitoring the bolts show that the flat washers cannot maintain the tension over an extended period of time. The Brevard (1) sign is a clear indication of this trend. Another observation is that the tension fluctuated more with the flat washers than the spring washers.

Installation procedure

The results also show that the current installation technique is faulty. In four out of seven cases, the bolts remained under-tensioned in spite of being installed with the proper torque according to FDOT specifications. In one case the bolt was over-tensioned despite being torqued to the proper FDOT specifications.

In this case, the tension in the bolts depends on the magnitude of the torque applied. The method of installation is described with detail in the FDOT manual [2]. However, we were surprised to find that the different FDOT jurisdictions are not in agreement over the value of torque to be used. Table 1 in the first part of this report shows that one jurisdiction at least (Orlando) uses a value of torque of 36 ft-lb, different from the 43 ft-lb used by the other FDOT offices.

Over the course of the study, a total of 31 installations of instrumented bolts with flat washers were performed with the conventional method. Table 19 summarizes the results of these installations. For each of the seven signs, the different installations are listed, with the corresponding dates and the values of the tension measurements for the two readings following each installation. We are including the second reading in the table, because we observed that sometimes, although the first readings indicated a value outside of the allowable tension range, after a short initial period of adjustment, the second and subsequent readings will fall between the range.

It can be seen that out of 11 cases, only 32% (or 10 cases) were successful installations, and 68% (or 4 cases) were unsuccessful. Successful means that the tension in the bolt was in the

allowable range. This is a fairly high rate of failure, specially compared to the 81% rate of success for the new torquing method, as described above. It was observed that lubrication of the connection prior to installation improves the chances of success. However, from our experience in the field with the FDOT crews, consistent lubrication of the connections does not seem a realistic option.

	Reading #	1	2
Sign	Date	(lb.)	(lb.)
Tampa	5/18/98	2915	3613
	6/10/98	4273	2959
	2/1/99	2414	
	2/8/99	2159	1787
Orlando	5/16/98	2601	9705
	6/10/98	2671	1992
	10/23/98	2140	
	11/4/98	1409	1243
	12/2/98	2404	1987
	2/1/99	2208	
	2/8/99	1705	
	2/15/99	1098	
	2/22/99	2674	
Brevard (2)	5/16/98	3523	5714
• •	6/1/98	6009	
	6/10/98	4177	1557
	11/13/98	3316	5291
	1/25/99	1165	777
	2/8/99	1547	1040
Brevard (1)	7/14/98	2651	2529
Sebastian Inlet	5/15/98	4215	3568
	12/4/98	3774	3780
	2/5/99	4300	3449
Indian River	5/15/98	2255	5810
	6/25/98	1672	1877
	11/6/98	6455	
	11/11/98	1239	
	12/9/98	970	672
	2/5/99	2199	2124
Martin County	5/15/98	3818	3267
•	12/4/98	3476	4722

Table 19: Tension in Bolts with Flat Washers

CONCLUSIONS AND RECOMMENDATIONS

Conclusions

A new method is proposed to measure and maintain the tension in the bolts of break-away slip base connections. The method is fairly simple to implement, and it is based on the utilization of Belleville spring washers. For each different bolt diameter and associated load range, a combination of spring washers is recommended. The stacks of spring washers, which come prewrapped, are installed on the bolt just as any ordinary washers, and the bolts are torqued with any kind of torque wrench. According to the method, a block of filler gages measures the deflection of the stack of spring washers which is proportional to the tension in the bolt. For each desired value of tension, there is a corresponding value of deflection. The main advantage of this technique is that it eliminates the uncertainties associated with torque measurements, and it provides a more reliable way to estimate the tension of the bolt.

Seven signs were selected along Central and South Florida highways. At each sign, one of the posts was equipped with bolts with spring washers, while the other post remained connected with bolts with conventional flat washers. At each sign, one of the bolts with spring washers and one of the bolts with flat washers were instrumented with strain gages, and the variations of tension in these bolts were monitored over a period of several months.

The following conclusions were obtained based on the results of the field monitoring.

- 1. Over the long term, bolts installed with spring washers appear to maintain the tension in the bolt. No instances of consistent bolt loosening below the allowable range of tensions were observed, even in high traffic areas like Orlando.
- 2. On the contrary, several bolts installed with flat washers exhibited a downward trend in the bolt tension.
- 3. The proposed installation procedure based on measuring deflections of the spring washers appears to be more successful than the conventional method based on measuring the torque. 81% of the bolts installed with spring washers had their initial tension in the allowable range, as opposed to only 32% of the bolts installed with flat washers.

- 4. The effectiveness of the break-away connection with spring washers was demonstrated during a car accident. A car hit a sign post equipped with spring washers, near the Sebastian Inlet. The break-away functioned perfectly and the post separated from the base.
- 5. Over the monitoring period, no significant advantage was observed regarding the use of stainless washers. Minor corrosion was observed for both the stainless and galvanized washers, and it appears to be due to the fact that the plastic wrapping of the stack of washers traps some humidity. The solution could be to slash the wrapping after installation.

Recommendations

Based on the results of the field monitoring, which confirm the results of the laboratory tests from the phase I of the project, spring washers appear to be a good alternative to the use of conventional flat washers, for break-away connections. The accurate measurement of the deflection of the stack of spring washers remains a critical issue. Filler gages can be used with a reasonable rate of success, as shown during this testing program, although the investigators feel that the FDOT crews would be reluctant to use the method as is. As a compromise, the bolts could still be installed with spring washers, to ensure that no loosening occurs, but they could be torqued with the conventional method.

To take full advantage of the proposed solution for break-away connections, we strongly recommend that a special measurement device be developed to specifically measure the deflection of the stack of spring washers. Such a device would significantly improve the efficiency of the method.

REFERENCES

- 1. FDOT, Ground Mounted Single And Multi-Pos Sign Inspection Manual, Florida Department of Transportation, November 1994.
- 2. Pinelli, Jean-Paul, Subramanian, Chelakara. Final Report, Study of Break-Away Sign Connections, State Job No. 99700-3344-119, Contract No. BA521, WPI No. 0510777, March 18, 1998