

2017 FDOT – Halls River Bridge FRP Workshop

May 3, 2017 Tampa, FL

HRB-FRP Workshop (Part 1)

Presenters:

Mamunur Siddiqui, EOR, FDOT D7

David Pelham, FDOT D7

Elisha Masseus, FDOT D6



Outline:

Part 1 – Bridge Design

- 1. Halls River Bridge Project Overview
- 2. Hybrid Composite Beams (HCB)
- 3. GFRP-Reinforced Concrete
- 4. Challenges
- 5. Lessons Learned





Project Overview – Halls River Bridge Replacement

Designer: FDOT District 7 Structures Design Office Bridge EOR: Mamunur Siddiqui, P.E. Bulkhead/Seawall EOR: Richard Hunter, P.E. (ACE) FDOT Developmental Standards EOR: Steven Nolan, P.E.



Owner & Maintaining Agency



Collaboration Research

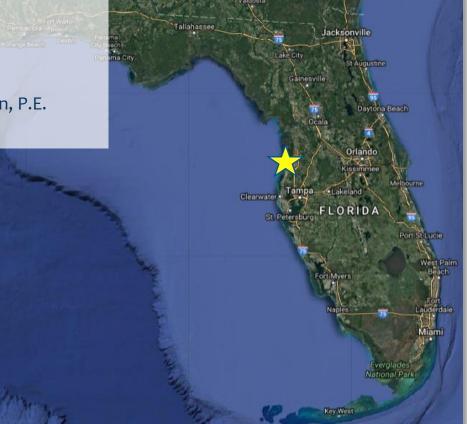
<u>HRB-FRP Workshop</u>

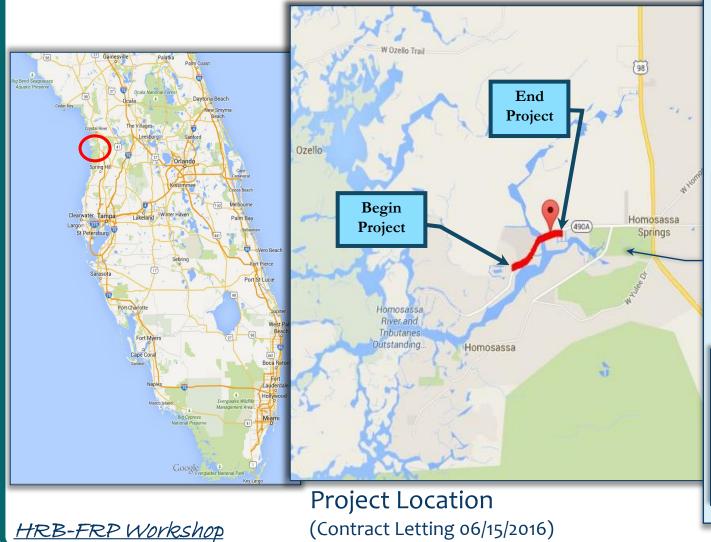
FDOT

Design & Bi-Annual Inspection



Funding & Oversight





Ellie Schiller Homosassa Springs Wildlife State Park



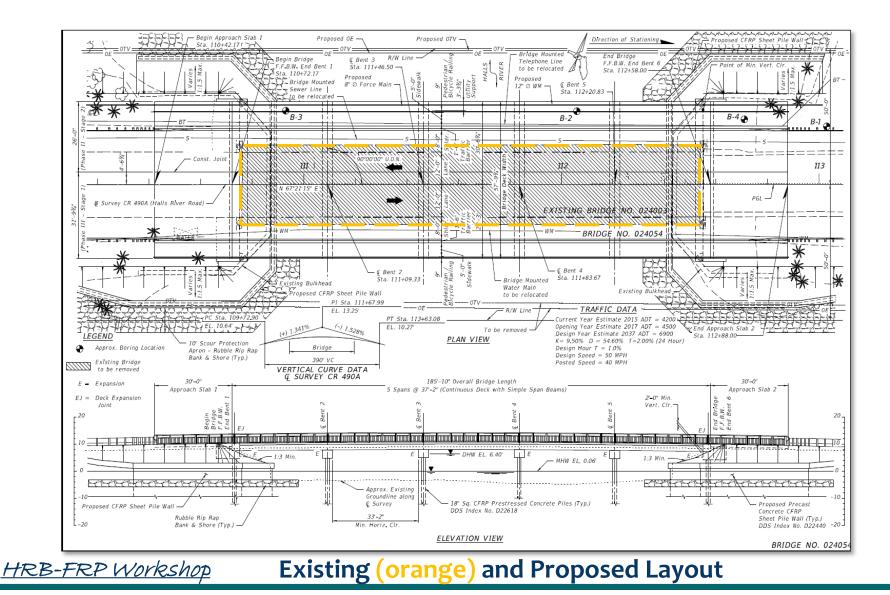
Wild Manatees reside in the park year round.

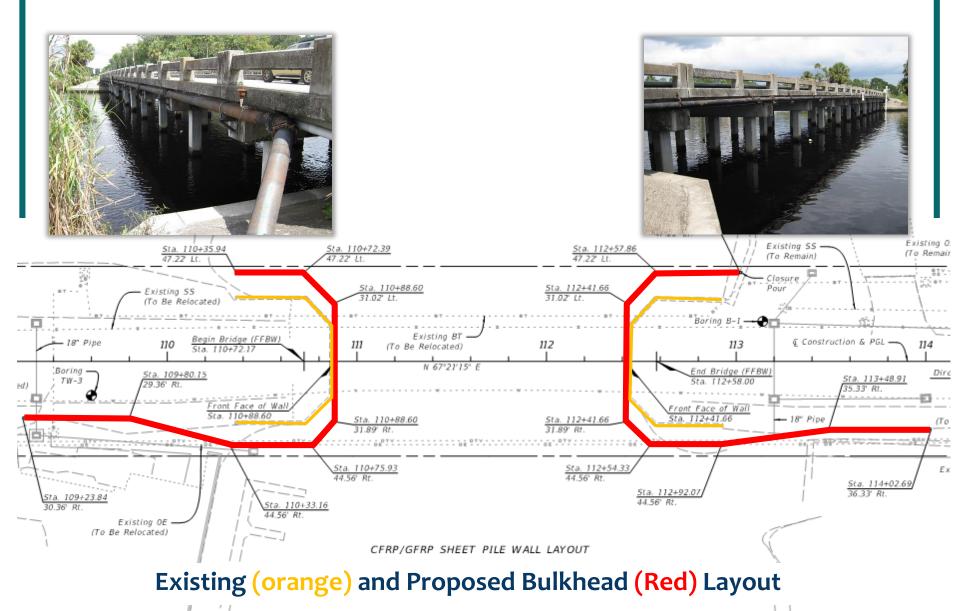


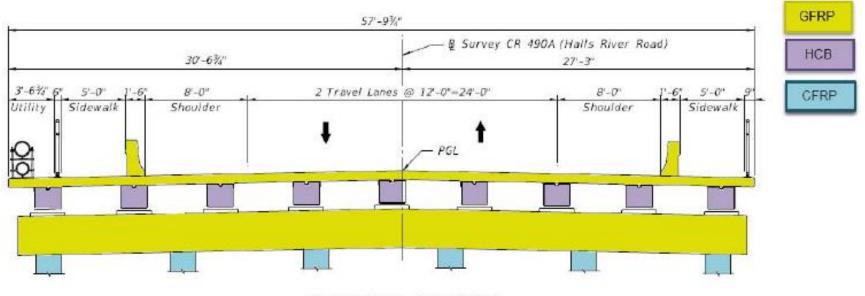
Lu the hippo, honorary citizen of Florida since 1991.



Vintage Postcard: "T.V.'S Gentle Ben makes his home at the Ivan Tors Animal Actors Training Academy here, and is on hand to greet visitors when not on filming location." Homosassa Springs.



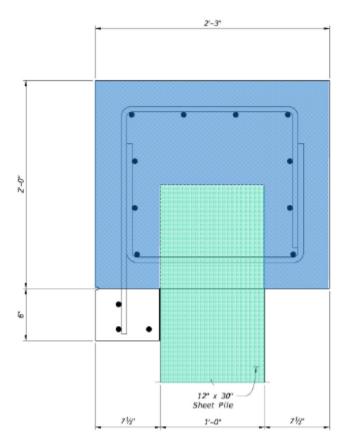




COMPLETED STRUCTURE



CFRP/GFRP Sheet Pile Walls





CFRP



- A. Overview
- B. Components
- C. Design
- D. Specifications





A. HCB Overview

- i. Proprietary Product background (Hillman Composite Bridge, Inc.)
- ii. Other Projects: DOT's (Maine, ...), Railroads ...



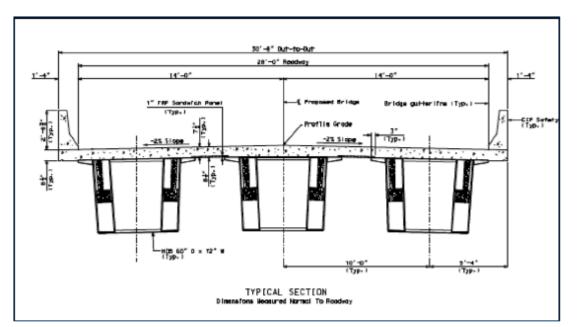
HRB-FRP Workshop

Source: HCB, Inc.

A. HCB Overview

Other Projects: Double Web Box HCB – Missouri

• 60 in depth, 120 ft long





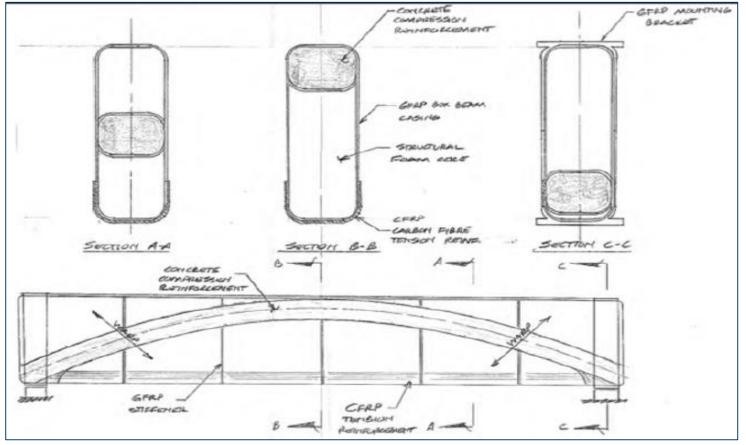
Source: HCB, Inc.

Source: HCB, Inc.



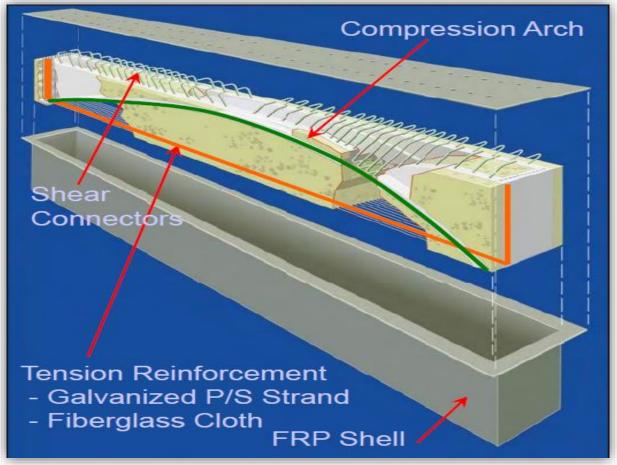
A. HCB Overview

Original Sketches – 1996



Source: HCB, Inc.

B. Components



HCB Design and Maintenance Manual (HCB, Inc.)



Hybrid Composite Beams (HCB) Components Β. Shear Stirrups Placed at 45 Degree Concrete Placed in Arch Conduit Profile **Two Prestressing Strands** Placed in Arch to Anchor Bottom Leg of Shear Connector Shear Stirrups Anchored by Strand Virginia Tech – Tide Mill Bridge (Shainur)

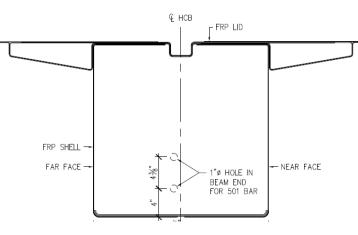


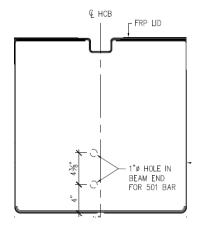
- B. Components
 - i. Wings vs. No wings
 - Savings in HCB costs
 - SIP Metal
 - hangers and overhang brackets for overhang forming
 - ii. CIP compression arch
 - Self-Consolidating Concrete (Dev346SCC)
 - Ease of HCB Transport, Erection and Installation





<u>RP Workshop</u>



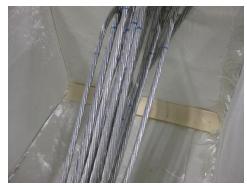


- B. Components
 - iii. Shell
 - Glass + resin matrix
 - 0°, 90° and ± 45° fibers orientation
 - Mechanical Properties



- Rules of mixture (micro-mechanics)Experimental (ASTM panels tests)
- iv. 0.5"ø, 7-wire low-lax galvanized strands,270-ksi, unstressed
- iii. Interface shear reinforcing
 - MMFX \rightarrow grade 60 \rightarrow zinc-coated (galvanized)







C. Design

- i. Beam cross section
- ii. Section Properties
- iii. Deflection
- iv. Camber
- v. Flexural Stress
- vi. Flexural Strength
- vii. Vertical Shear
- viii. Horizontal Shear
- ix. Fatigue

HRB-FRP Workshop

x. Load Rating

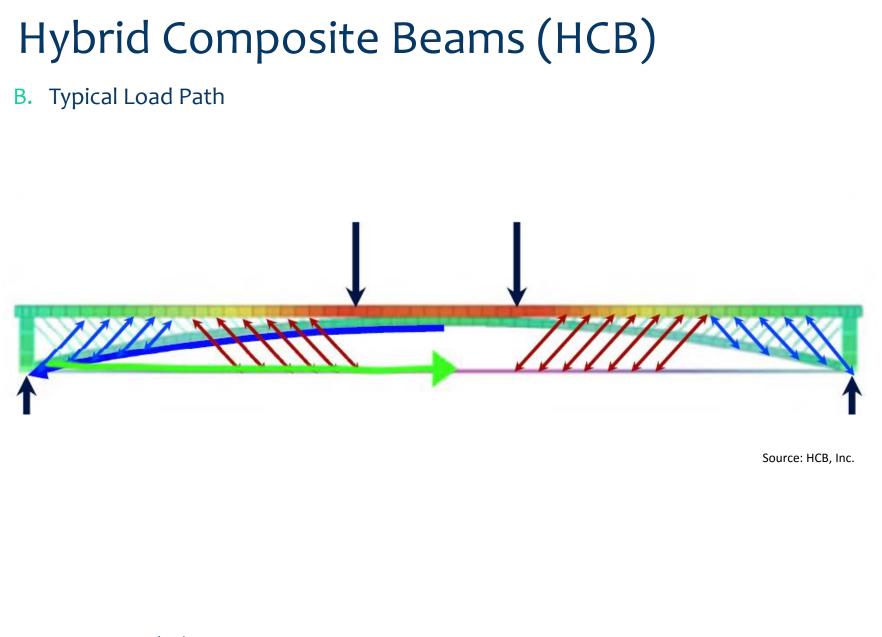
Hybrid-Composite Beam (HCB[®]) Design and Maintenance Manual



RTE 205 (RIDGE RD.) Over Tide Mill Stream, Westmoreland Co. State Project No.: 0205-096-101, B601 Federal Aid Project No.: BR-096-6(015) NBIS No. 27818

Prepared for The Virginia Department of Transportation

> John R. Hillman, PE, SE HCB, Inc.



C. Design

- i. Beam cross section
 - Depth: span/18 to span/25
 - Width: depth/3 to depth/2
 - Coordinate with HCB, Inc.
- ii. Section Properties
 - 10 points along beam
 - All components transformed to equivalent FRP web

iii. Deflection

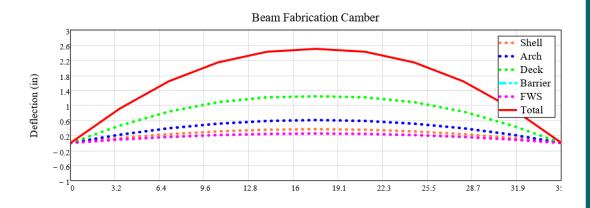
- Serviceability governs → LL deflections
- FRP materials \rightarrow low elastic modulus \rightarrow larger deflections
- Concrete + steel \rightarrow used to meet deflection criteria

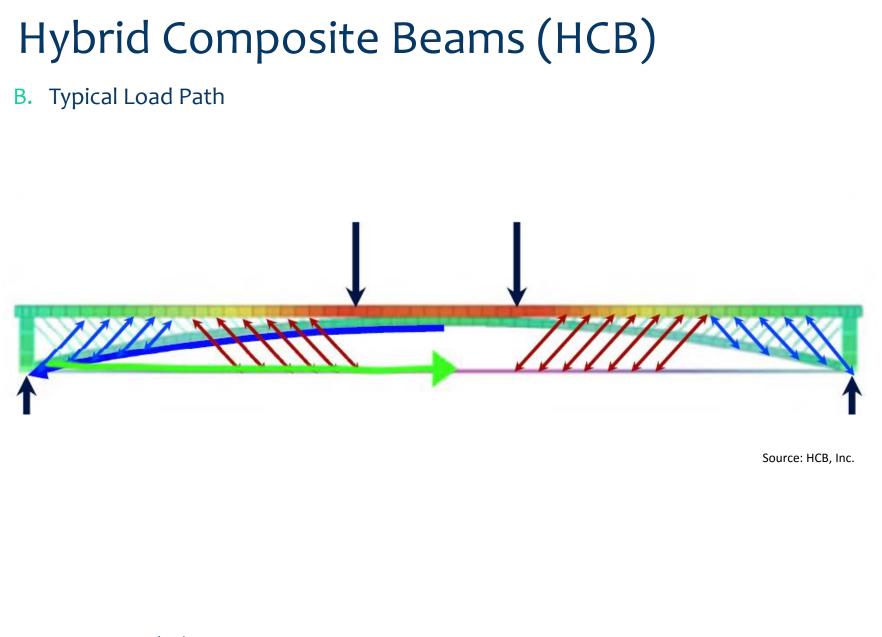
C. Design

- iv. Camber
 - Total = $^{\text{shell}}$ DL.camber_i + $^{\text{arch}}$ DL.camber_i + $^{\text{Deck}}$ + $^{\text{A}}$ SDL_i + $^{\text{A}}$ allow_i
 - Allowance = 0.25" (creep and net positive camber)
 - Check arch pour and deck casting stages
- v. Flexural Stress

2P Workshop

- Use individual moment inertia of each material
- Check construction stages: FRP shell, SW, SDL, LL
- Check service conditions





C. Design

- vi. Flexural Strength
 - Over-reinforced → required steel from deflection criteria
 - Strain compatibility method \rightarrow moment strength
 - Additional $\varphi = 0.9 \rightarrow$ FRP laminates (ACI 440.1)

vii. Vertical Shear \rightarrow Virginia Tech Research

- Demand:
 - FRP Shell + Concrete
- Capacity:
 - FRP web
 - Concrete
- Beam casting check for FRP webs





C. Design

- viii. Horizontal Shear
 - ACI Shear-Friction Method (ACI 318, 11.6.4)
 - Connector capacity
 - Coefficient of friction \rightarrow steel beam with headed studs
 - > 0.6 (roughness of FRP, conservative)
 - Angle of inclination of shear connectors
- ix. Fatigue
 - Tested to Class 1 freight rail traffic \rightarrow 500,000 to 2,000,000 cycles
 - FRP and steel \rightarrow Tensile stress ranges 10% ultimate capacities
- x. Load Rating
 - Similar to reinforced concrete beams

D. Technical Special Provisions(TSP)

TECHNICAL SPECIAL PROVISION

FOR

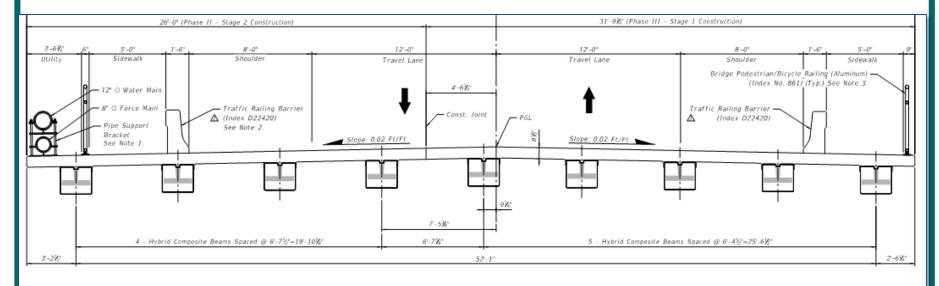
SECTION T450 - FURNISHING & INSTALLING HYBRID-COMPOSITE BEAMS

FINANCIAL PROJECT ID: 430021-1-52-01

The official record of this Technical Special Provision has been electronically signed and sealed using a Digital Signature as required by Rule 61G 15-23.004, F.A.C. Printed copies of this document are not considered signed and sealed and the signature must be verified on an electronic copies.

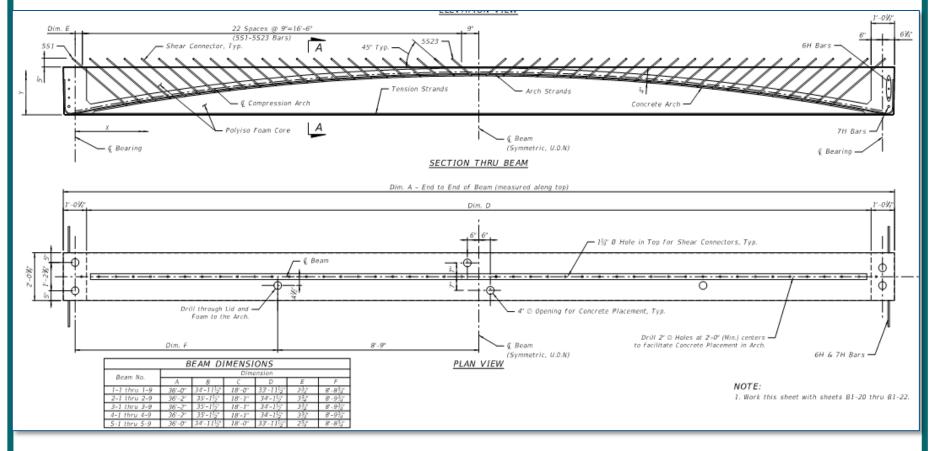
Professional Engineer: Mamunur Rashid Siddiqui, P.E. Date: March 3, 2016 Fla. License No.: 70094 Firm Name: FDOT Firm Address: 11201 N McKinley Dr. City: Tampa, State: FL, Zip code: 33612 Certificate of Authorization: N/A. Pages: 1-13

Example Plan Sheet Details:



TYPICAL SECTION THRU BRIDGE DECK

Example Plan Sheet Details (cont.):





- A. Overview
- B. Components
- C. Design
- D. References, Codes and Specifications



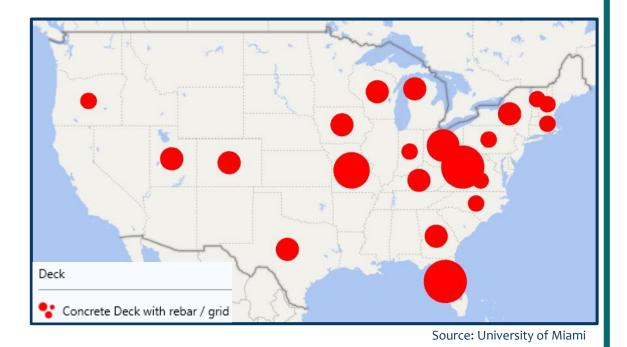


(Photographs) Hughes Bros. GFRP Bars.





- A. Overview
 - i. Background
 - ii. Other projects (sourced from ACMA)
 - i. 67+ USA
 - ii. 200+ Canada

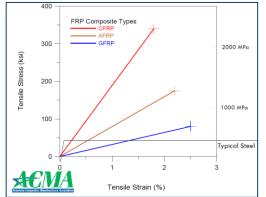


- B. Components
 - i. RC Deck
 - ii. RC Bent Cap
 - iii. RC Back and Wing Walls
 - iv. RC Diaphragms





- C. Design Principles:
 - i. Based on
 - Equilibrium
 - Compatibility of Strains
 - Stress-strain characteristics of the material
 - ii. Brittle behavior of the FRP reinforcement



- i. Linear-to-failure stress-strain relationships must be used
- ii. Design mainly controlled by crack width, bond factor k_b or min reinforcement requirement, w_{lim} = 0.02 inch, k_b =0.9 to 1.4



- C. Design Failure Mode:
 - i. If the FRP reinforcement ruptures, sudden and catastrophic failure can occur
 - ii. Concrete crushing has typically been considered
 - iii. The margin of safety against failure for FRP is higher than the conventional steel,
 - FRP reinforced sections are designed based on required strength considerations and check for creep rupture stress limits, and serviceability criteria

- C. Design FRP Rebar:
 - i. A direct substitution between FRP and steel reinforcement is not possible
 - differences in the mechanical properties of the two materials
 - f_{fu} =70ksi to 110ksi with reduction (Ce=0.7) used $f_{fd.pr.slab}$ =63ksi
 - ii. Modulus of elasticity much lower than steel, $E_f = 6500$ ksi
 - iii. FRP reinforced concrete sections do not show ductility
 - i. Steel yield first \rightarrow Ductile failure mode
 - iv. Resistance Factors:
 - Flexure and tension: $\phi_{f} = 0.55$ to 0.65
 - Shear and torsion: $\phi_{\rm v} = 0.75$

- C. Design Deck (AASHTO LRFD)-2009
 - Design Tensile strength and strain

$$f_{fd} = C_E f_{fu}$$
$$\varepsilon_{fd} = C_E \varepsilon_{fu}$$

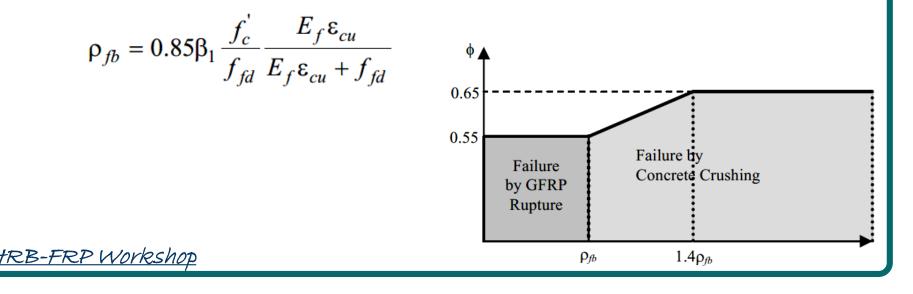
Table 2.6.1.2-1—Environmental Reduction Factors

	Environmental
Exposure	Reduction
Condition	Factor, C_E
Concrete not exposed to earth and weather	0.80
Concrete exposed to earth and weather	0.70

C. Design - Deck and Cap

Resistance Factors

 $\phi = \begin{cases} 0.55 & \text{for } \rho_f \le \rho_{fb} \\ 0.3 + 0.25 \frac{\rho_f}{\rho_{fb}} & \text{for } \rho_{fb} < \rho_f < 1.4 \rho_{fb} \\ 0.65 & \text{for } \rho_f \ge 1.4 \rho_{fb} \end{cases}$



C. Design - Deck and Cap (ACI, AASHTO LRFD)

Flexural Resistance (Rectangular sections)

When
$$\rho_f > \rho_{fb}$$

$$a = \frac{A_f f_f}{0.85 f_c' b} \qquad f_f = \sqrt{\frac{\left(E_f \varepsilon_{cu}\right)^2}{4} + \frac{0.85 \beta_1 f_c'}{\rho_f} E_f \varepsilon_{cu}} - 0.5 E_f \varepsilon_{cu} \le f_{fd}$$

$$M_n = A_f f_f \left(d - \frac{a}{2}\right)$$

When $\rho_f < \rho_{fb}$

$$c_b = \left(\frac{\varepsilon_{cu}}{\varepsilon_{cu} + \varepsilon_{fd}}\right) d \qquad f_{fd} = C_E f_{fu}$$

$$M_n = A_f f_{fd} \left(d - \frac{\beta_1 c_b}{2} \right)$$

C. Design - Cap (ACI 440.1R)➢ Shear Resistance

$$V_n = V_c + V_f$$

$$V_c = 0.16\sqrt{f'_c b_w c} \quad \text{not be larger than } 0.32\sqrt{f'_c b_0 c}$$

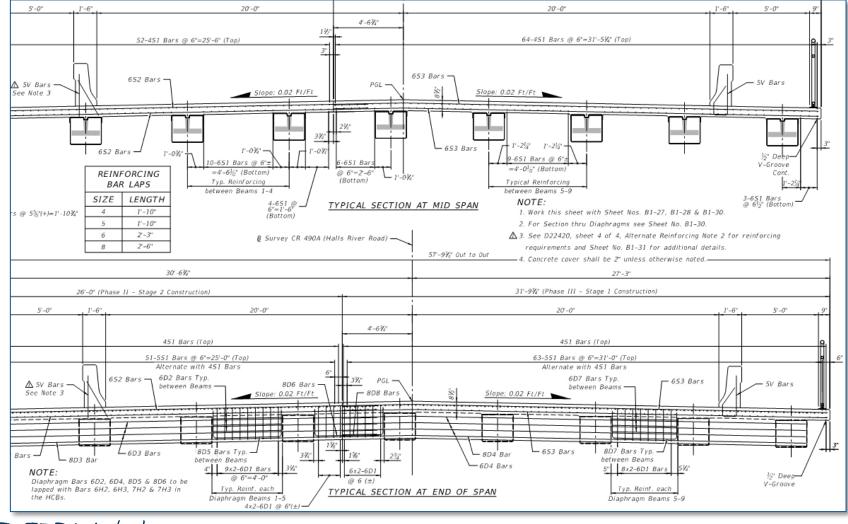
$$V_f = \frac{A_{fv} f_{fv} d}{s}$$

$$f_{fv} = 0.004E_f \le f_{fb}$$

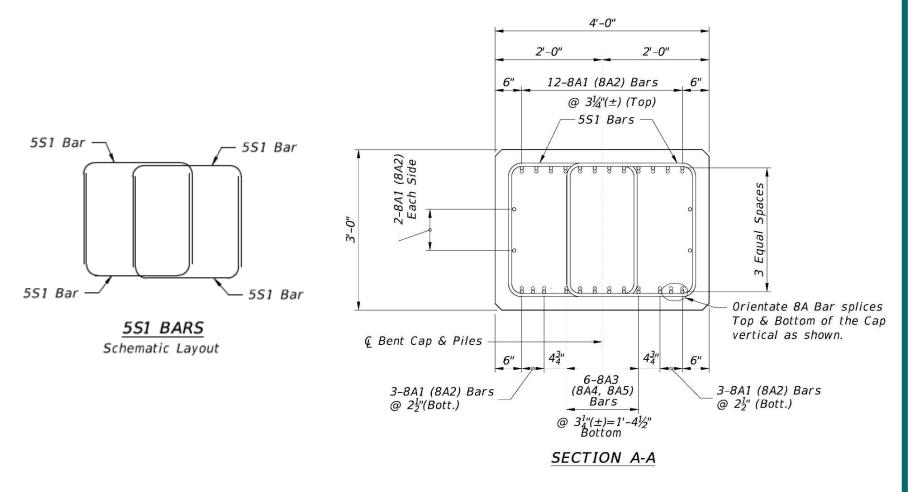
$$f_{fb} = \left(0.05\frac{r_b}{d_b} + 0.3\right) f_{fd} \le f_{fd}$$

GFRP -**RC** Deck

Plan Sheet Details:



Plan Sheet Details:



- D. References, Codes and Specifications
 - i. There are a limited number of standards and codes
 - ii. The lack of accepted design guidelines and code language
 - iii. Design Manuals:
 - ACI 440.1R

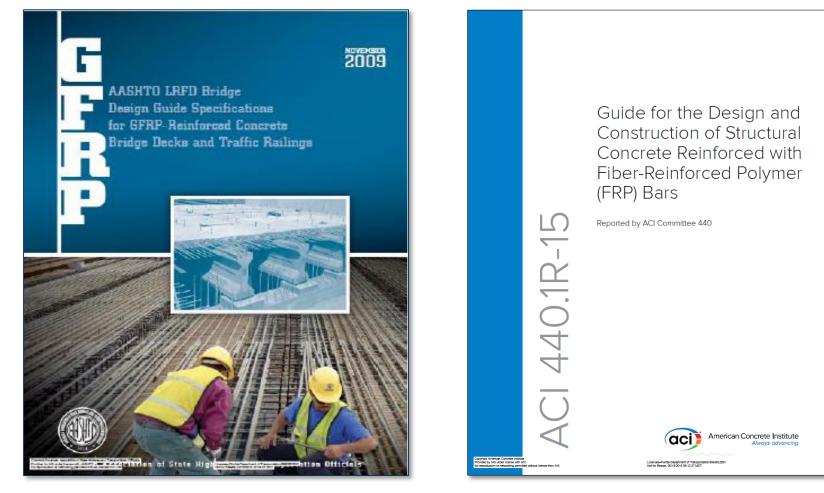
Guide for the Design and Construction of Structural Concrete Reinforced with Fiber-Reinforced Polymer

AASHTO LRFD GFRP-2009

Design Guide Specifications for GFRP-Reinforced Concrete Bridge Decks and Traffic Railings



D. References, Codes and Specifications



- D. References, Codes and Specifications
 - iv. FDOT Manuals and Standards:
 - FDOT SM Volume 4
 - Fiber Reinforced Polymer Guidelines (FRPG)
 - Material Manual Section 12.1 Volume II
 - > Fiber Reinforced Polymer Composites
 - Developmental Design Standards
 - Square Piles, Sheet Piles, Traffic Railings, Approach Slabs, Bar Bending Details
 - v. FDOT Developmental Specifications:
 - Dev932
 - Nonmetallic Accessory Materials for Concrete Pavement and Concrete Structures
 - Dev933
 - Prestressing Strand (CFRP)

D. References, Codes and Specifications

Specifications and Estimates/Specifications/ Materials Manual Section 12.1, Volume II

FIBER REINFORCED POLYMER COMPOSITES

Section 12.1, Volume II



STRUCTURES DIVISION

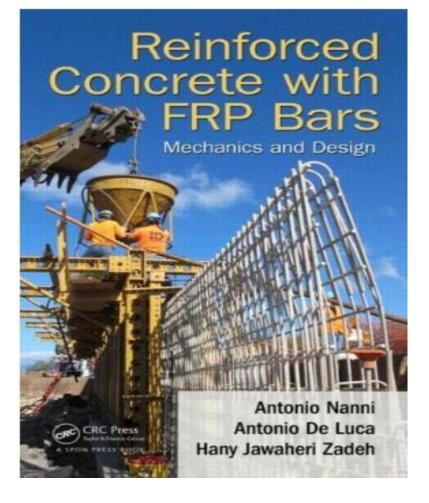
FLORIDA DEPARTMENT OF TRANSPORTATION

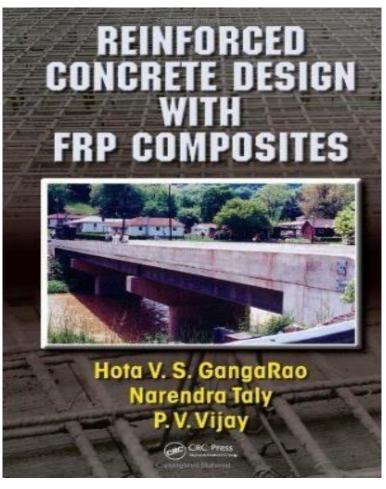
FIBER REINFORCED POLYMER GUIDELINES (FRPG)

FDOT STRUCTURES MANUAL VOLUME 4 JANUARY 2016



D. References, Codes and Specifications





4. Challenges



Challenges

A. HCB

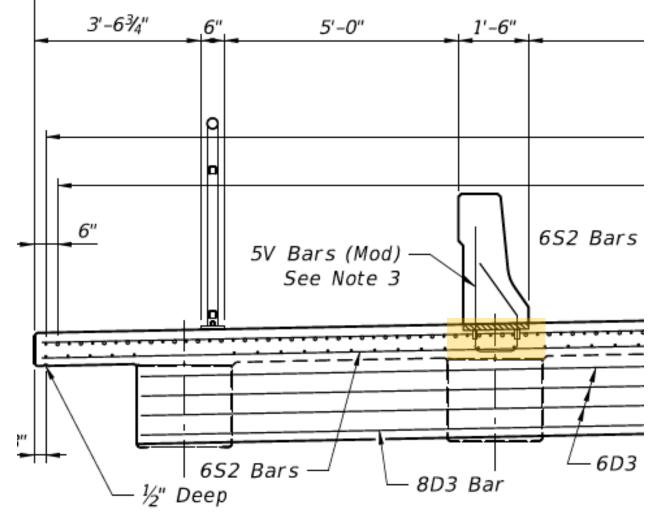
- i. Proprietary product
- ii. Design Criteria
- iii. Inspection for closed system
- iv. Durability verification
- v. Fabrication QA/QC
- B. GFRP Reinforced Concrete
 - i. Lap Splice: deck, cap, and diaphragm
 - ii. Rebar unit price
 - iii. Reinforcing Bar List
- C. Funding and Costs
 - i. FHWA and County



REINFORCING BAR LAPS	
SIZE	LENGTH
4	1'-10"
5	1'-10"
6	2'-3"
8	2'-6"

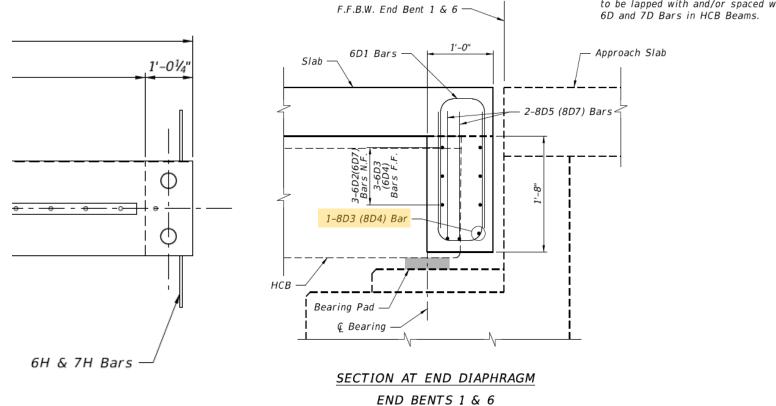
Challenges

Plan Sheet Details post Installed Traffic Railing:



Challenges

Diaphragms Plan Sheet Details:



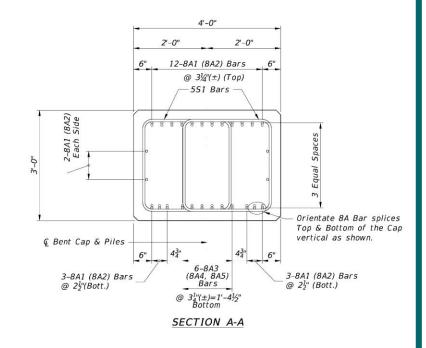
Horizontal Diaphragm 6D and 8D Bars to be lapped with and/or spaced with 6D and 7D Bars in HCB Beams.

5. Lessons Learned



Lessons Learned

- To develop standard details and specifications
- Design for Phase Construction
- Rebar arrangement no mechanical coupler
- Lead time, Sole source of CFCC (Tokyo Rope)
- HCB QA/QC plan
- Sheet pile wall driving
- Pile capacity



Summary

- Demonstration Project with Innovative Materials First in Florida
 - ✓ Superstructure: Hybrid Composite Beams; GFRP Bars: Deck, Barriers & Approach Slabs
 - ✓ Substructure: CFRP Pre-stressed Piles; Bent Caps: GFRP Bars
 - ✓ Sheet Pile Walls: CFRP/GFRP Sheet Piles; Wall Cap: GFRP Bars
- Contractor Bid Cost \$6.016 Million (Structures = \$4.06 Million)
 - Bridge Cost = \$218 / sq. ft.

(Conventional Construction = \$166 / sq. ft.)

- Accelerated Construction
 - Lighter Materials Beams and Rebar
 - Faster Transportation and Delivery reduced construction time

Questions?





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David Pelham

Senior Structures Designer/DW Geotech PM FDOT , District 7 (813) 975-6771