esign Training Post-Expo 6/15/2016, 1:00pm - 4:00pm Hilton Daytona Beach Oceanfront Resort

GFRP Rebar Workshop

2016

St Johns Room

100 North Atlantic Avenue Daytona Beach, Florida 32118, USA Tel: 1-386-254-8200



Part 2 - Discussion on Needs



- 1. Design Criteria
- 2. Durability Issues
- 3. Material Specifications & Vendor Approvals
- 4. Construction Specifications
- 5. Products in the marketplace (present and future)
- 6. Research Focus





- Barriers to expanded FRP Implementation
- Potential Focus Areas



FDOT's Fiber-Reinforced Polymer Deployment Train



GFRP Reinforcing CFRP Prestressed Piles Fender Systems External FRP Laminate Repairs





Barriers to expanded FRP Implementation:

- 1. First cost
 - This topic has to be addressed by industry directly, but volume of material was identified as the main driver. Expanding the number of potential structural element uses in the *FRPG* would help to increase the volume.
 - First cost should include benefits of reduced cover, reduction of additives, no need for surface coating, and labor/installation savings due to lightweight.
 - SEACON will generate LCC data that may be helpful.
 - Consider example cost comparison similar to that prepared for SMO on SS/SS clad rebar.





Barriers to expanded FRP Implementation (cont.):

- Lack of confidence in durability for submerged environments (FDOT seeking 75

 100 year service life)
 - Accelerated testing could address this issue. OC can volunteer its laboratory for samples subjected to sustained load+saltwater+60°C ? (alkalinity). The outcome could be a new set of creep-rupture curves that account for environmental effects. Initially proposed to look at existing data through a synthesis study.
 - OC to look into their experimental capabilities when using naked #3 bars (Nanni proposal).
 - Look at quality of bends compared to straight bars.
 - Compression testing will eventually have to be addressed.





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Roadmap for Safe Deployment of FRP **Reinforcement for Concrete Structures**

Barriers to expanded FRP Implementation (cont.):

- 3. Limitations on the strength due to degradation of properties over time (currently C_F factor = 0.7 for GFRP exterior environments) [goes with item #2]
 - Use tests on field-retrieved bars and correlate to accelerate-conditioning tests to develop knockdown factors for 100 years of service life. Initially proposed to look at existing data through a synthesis study (see Item 2).
 - Existing stress limit is 0.20 of guaranteed times C_F to account for creeprupture and fatigue under service loads. *Is the creep rupture limit actually* affected by long-term environmental exposure?
 - Current FDOT Materials research project: *BDV34 977-05* Degradation Mechanism and Service Life Estimation of FRP Concrete Reinforcements. SSDO expressed desire to implement waterline applications of GFRP based on partial positive results at some low risk locations.

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Systems

External FRP

Laminate

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Roadmap for Safe Deployment of FRP **Reinforcement for Concrete Structures**

Barriers to expanded FRP Implementation (cont.):

- 4. Limitations on strength due to low design resistance factors (phi factors) related to lack of ductility and strength variability in the FRP materials (currently 0.55-0.65 for tensioned-control to compression-controlled flexural failure modes)
 - This is a designer's issue that could be tackled immediately based on reliability.
 - Separate shear from flexure.

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For flexure, revisit existing data and reconfirm proposal by Jawaheri and Nanni (see Table 9).

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Table 9—Recommended strength reduction factors for FRP reinforced beams

Limit state	Strength reduction factor (ϕ)
FRP rupture*	0.70
Concrete crushing*	0.75
Shear^\dagger	0.75

*Conservatively: $\phi = 0.70$ for both modes; †Shear reinforcement limit is modified as $V_f \leq 3V_c$.

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4. Limitations on strength... (continued)

Code comparison prepared by SSDO:

<u>Action</u>	Failure Mode	<u>Phi (AASHTO)</u>	<u>Phi (ACI)</u>	<u>Comment</u>
Conventional S	teel Reinforcing:			
Shear	Brittle	0.75	0.75	
Plain Concrete	Brittle	N/A	0.60	
Flexure-CC	Brittle	0.75	0.75	
Flexure-TC	Ductile	0.90 (1.00)	0.90	() = prestressed
FRP Reinforcing	g:	(<u>AASHTO-GS)</u>	<u>(ACI -440)</u>	
FRP Reinforcing Shear	g: Brittle	(<u>AASHTO-GS)</u> 0.75	<u>(ACI -440)</u> 0.75	
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Shear	Brittle	0.75	0.75 0.60	non-prestressed
Shear Plain Concrete	Brittle Brittle	0.75 N/A	0.75 0.60 <mark>0.65</mark>	non-prestressed non-prestressed
Shear Plain Concrete Flexure-CC	Brittle Brittle Brittle	0.75 N/A N/A	0.75 0.60 <mark>0.65</mark>	
Shear Plain Concrete Flexure-CC Flexure-TC	Brittle Brittle Brittle Brittle	0.75 N/A N/A N/A	0.75 0.60 0.65 0.55 0.65	non-prestressed

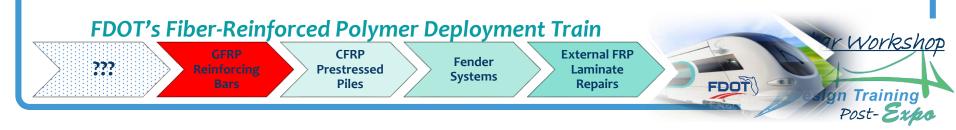
• Consider changing paradigm by looking at strain in GFRP as per steel (i.e., not allowed below a strain of 0.004, full 0.9 above a strain of 0.005).

FDOT's Fiber-Reinfo			
GFRP Reinforcing Bars	CFRP Prestressed Piles Fender Systems	External FRP Laminate Repairs	FDOT Son Training Post- Expo



Barriers to expanded FRP Implementation (cont.):

- 5. Restrictions in bar bending capabilities, and challenges with field modifications to bar shapes
 - This may be a perceived barrier. Manufacturers need to propose standardized shape of higher quality revisiting minimum radius of curvature and 60% efficiency.
 - Continuous close stirrups/ties are possible and allow sharp corners, and do not rely on GFRP-concrete bond. *Would test methods differ for these types of stirrups?*
 - FRP Bar Bending Index D21310 could be updated to address different bend radii for different production methods, but this must be tied to specification identification and acceptance criteria.





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Roadmap for Safe Deployment of FRP Reinforcement for Concrete Structures

Barriers to expanded FRP Implementation (cont.):

6. Low Elastic Modulus, resulting in greater deflections and larger crack openings

- This is a shortcoming with no immediate solution that industry may or may not consider addressing.
- Consider combining with FRC to control crack size openings and possibly deflections. Need tools to quantify effect of FRC on crack width and/or deflections.
- Consider combining GFRP stirrups/ties and carbon steel strand in PC applications
- Review Canadian codes crack width criteria
- Review relevance of Kb factor in ACI for GFRP
- 7. Update AASHTO Guide Specification
 - This is a shortcoming for state DOTs

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Roadmap for Safe Deployment of FRP **Reinforcement for Concrete Structures**

Potential Focus Areas:

- 1. Rationalization of Resistance Factors (phi factors) used to address lack of ductility and variability in material strength properties;
- 2. Refinement of Environmental Reduction factors (C_{F});
- **Resolution of durability question in submerged environments;** 3.
- 4. Advancement in bent bar fabrication;
- **Mitigation of lower elastic modulus effects** as related to member deflections 5. and concrete crack widths;
- 6. Investigate hybrid designs using FRC and/or Carbon-steel strand with GFRP rebar:

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Concrete Sheet Piles;

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7. Improved FRP Industry coordination especially between ACMA-TSC and AASHTO SCOBS-T6 (FRP) & T10 (Concrete);

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Roadmap for Safe Deployment of FRP **Reinforcement for Concrete Structures**

Potential Focus Areas (cont.):

8. Continued Standardization through:

- **Design Specifications** i.
 - AASHTO Guide Spec update (T6) -> LRFD Chapter 5 inclusion (T10);
 - ACI 318-GFRP design companion document/address column design;

ii. Material Specifications

- FDOT Specification Sections 932 & 933;
- ACI 440-K/ASTM D30.10: new Specification for Solid Round Glass Fiber Reinforced Polymer Bars for Concrete Reinforcement, WK43339;

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Pre-Fabrication iii.

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- Cages (ACP, Sheet Piles, Traffic Railings, Precast Caps)
 - Bespoke (wound stirrups & confinement);
- 2D-Grids/Mats (e.g. Decks and Noise Wall Panels);
- Bends/Stirrups/Hoops;
- Headed Anchors;
- Pre-designed of Structural Elements (such as **FDOT** <u>Design Standards</u> iv. Indexes);

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Possible Pendulum Testing of GFRP reinforced Traffic Barriers.

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Piles



Roadmap for Safe Deployment of FRP Reinforcement for Concrete Structures **Potential Focus Areas** (cont.):

9. Accommodation of potential customization and optimization of FRP reinforcing and other products

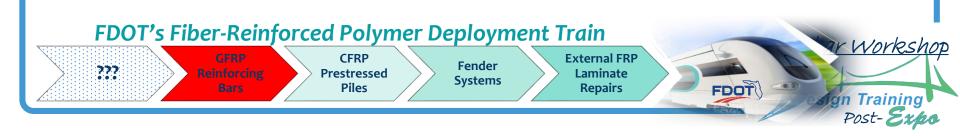




Potential Focus Areas (cont.):

10. Guidance on the use of Life Cycle Cost Analysis for FRP justification:

- i. Coordinate with SEACON-WP6;
- ii. Utilize FHWA/& NCHRP Report 483;
- iii. Consider Leveraging Sustainability angle:
 - From **2016 National Bridge Conference**: Jianwei Huang and Chris Strazar, "Sustainability of GFRP RC Bridge Deck: Materials Cost", Southern Illinois University Edwardsville: This research clarifies the concern of the high initial cost for GFRP RC bridge deck as compared to conventional steel RC deck;
 - **USDOT to require emissions-reduction goals for funding recipients** The US Department of Transportation is working on plans to require highway and transportation funding recipients to set and track carbon dioxide emissions-reduction goals as a condition of receiving money;
 - **FHWA proposal: Emissions could gauge success of transportation projects** The amount of emissions, along with congestion, traffic reliability and freight movement, could be used to evaluate the success of a transportation project under <u>new rules proposed</u> by the Federal Highway Administration. The agency has started a 90-day comment period in the proposal.





Potential Focus Areas (cont.):

11. Project Monitoring

- SMO monitoring Cedar Key Bulkhead rehab Test Beams under cap (3 surface coatings of GFRP);
- ii. FSU-UM monitoring Halls River bulkheads, piles, bent caps and deck Test beams under bulkhead (GFRP, CFRP, BFRP & SS);
- iii. Coordinate with FHWA for monitoring FRP under *Fixing America's Surface Transportation (FAST) Act*.

12. Outreach and Technology Transfer:

- i. FDOT Invitation to Innovation-FRP website;
- ii. FDOT Design Expo;
- iii. Project Case-Studies & Workshops.
- **13.** Repair Methods

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14. Bridge Inspection



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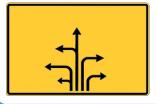
Part 3 - Action Items & Next Step

1. Immediate Action Items:

- Meet 2 or 3 times per year to meet initial deployment challenges;
- Review current durability data to address GFRP in the splash zone or submerged conditions, and refinement of 0.7 C_E factor;
- Address Anchorages and Mechanical Splices for GFRP;
- Update FDOT FRP Bar Bend Index D21310 with input from industry partners;
- Review 60% Bar Bend Capacity criteria;
- ACMA-RBMC to propose updates to AASHTO Guide Spec.

2. Next Steps

- Organize next meeting for December/January in South Florida;
- Continue development of FRP Rebar Roadmap;
- Explore funding and time requirements for C_E factor review.







Adjourn - Thanks for Coming !!



