FDOT/Concrete Coalition of Florida

Pilot Project for Maximum Heat of Mass Concrete - Research Highlights

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February 27, 2013

Background

- The FDOT currently defines mass concrete as:
 - Minimum dimension ≥ 3'
 - Ratio of concrete volume to the surface area >1'
- Requires a temperature differential control plan
- Model procedure outlined in ACI 207, also known as the Schmidt Method.

Scope of Research

- Use finite element modeling (FEM) to predict the temperature differentials and peak temperatures in massive concrete structures.
- Determine/Suggest the size of the element that does not need a mass concrete plan
- Determine the R values for soil

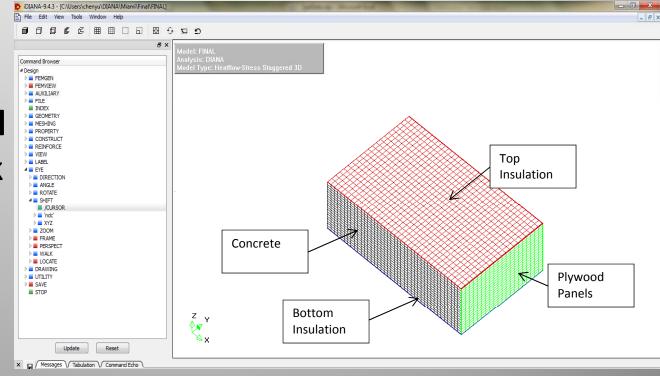
Finite Element Modeling (FEM)

- TNO Diana FEM program
- Adiabatic* temperature rise based on the ratio of cementitious components in a mix design
- Determined from Isothermal Calorimetry
 Testing

^{*}temperature rise without the loss or gain of heat to/from external sources

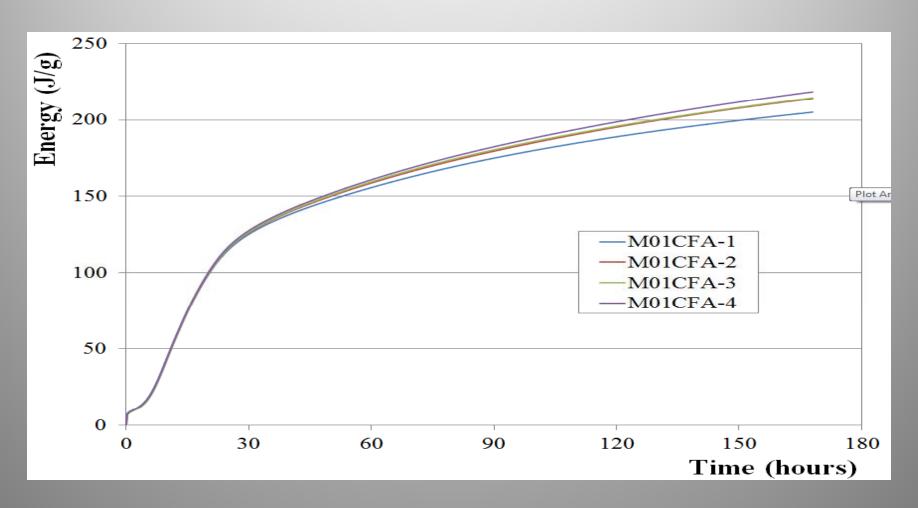
Finite Element Modeling (FEM)

 To construct the model, the hydrating concrete, as well as the formwork and insulating materials are taken into consideration.



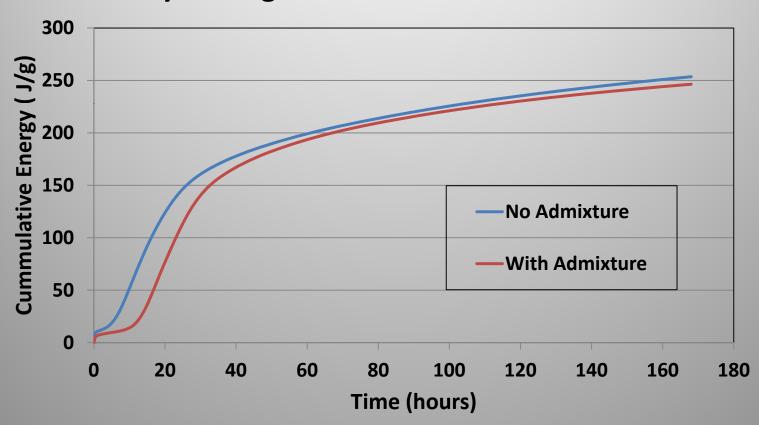
Laboratory Testing

 Hydration Energy curve obtained from Isothermal Calorimetry Testing



Laboratory Testing

 Hydration Energy curve obtained from Isothermal Calorimetry Testing



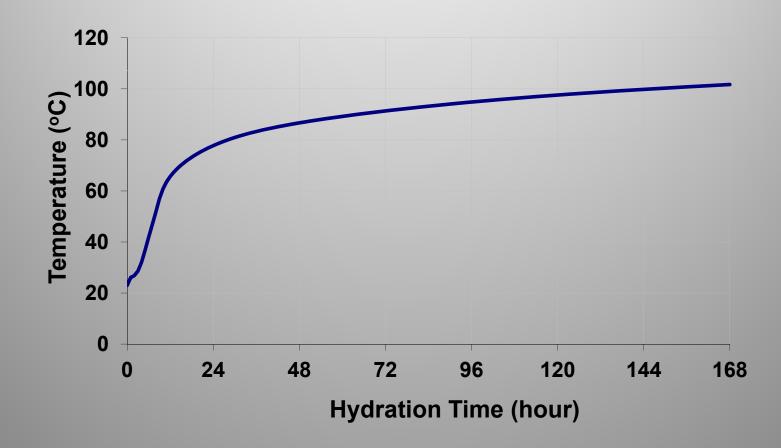
Type D – Water Reducer Retarder

Type F – high range water reducer

Air Entraining

Laboratory Testing

 Adiabatic Temperature Rise curve obtained from the Hydration Energy Curve



- Two bridge elements constructed in the field in Florida were monitored for temperature developments
 - Footing at a project in Miami
 - □ Footing at a project in Orlando

- Miami, Florida
 - Concrete Mix Design:
 - Portland Cement Type I/II
 - Fly Ash Class F

Miami, Florida



Contractor opted to place concrete directly on top of the soil

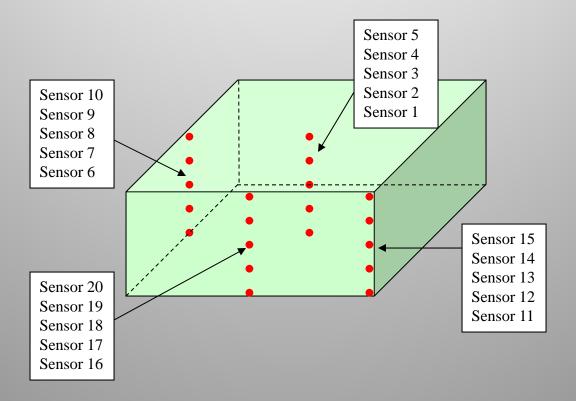
Installation of temperature sensors into the footing cage



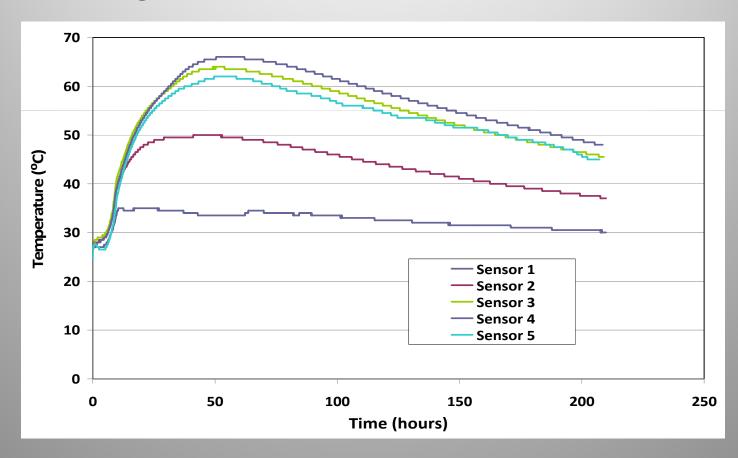
Temperature sensor



Location of temperature sensors in pier footing



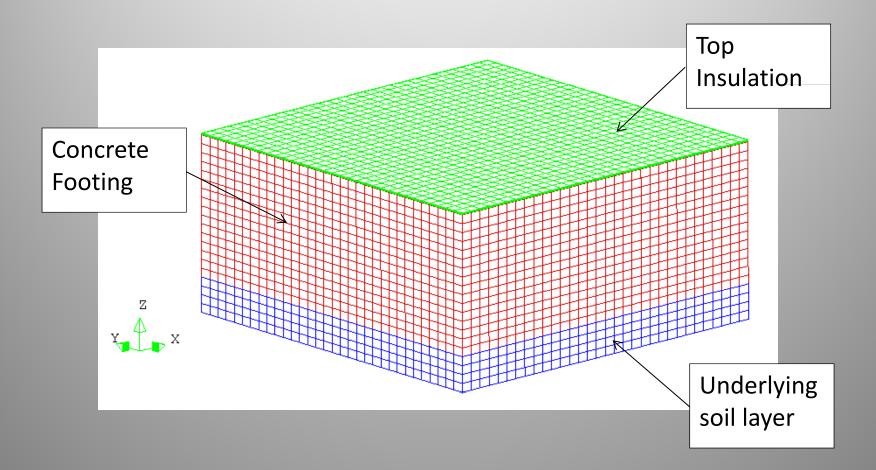
Footing, Miami, FL



Measured temperatures along vertical centerline of footing

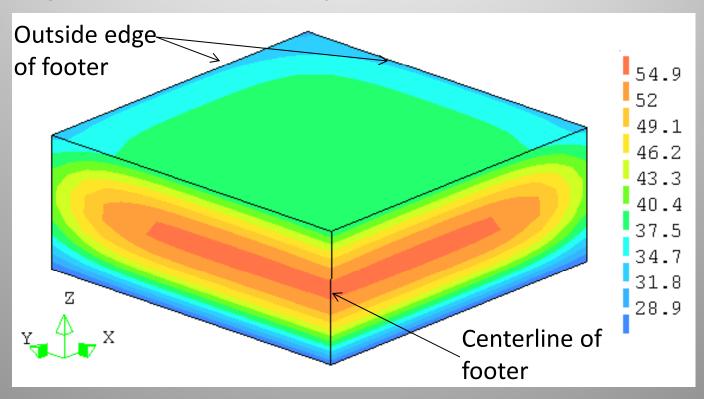
Modeling

Finite element mesh

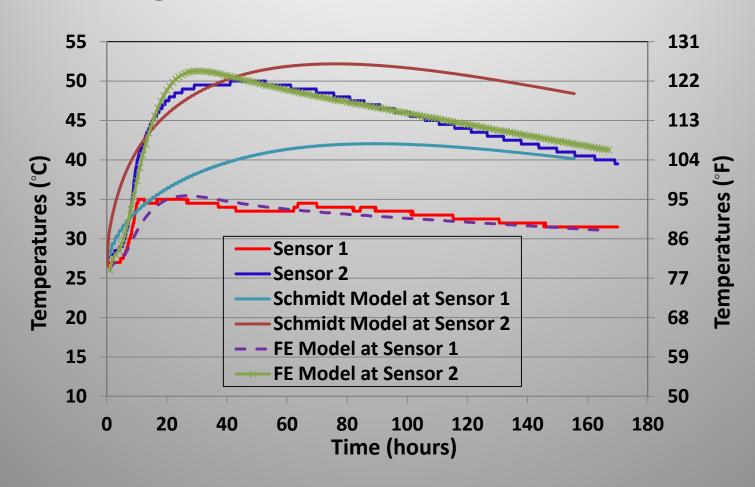


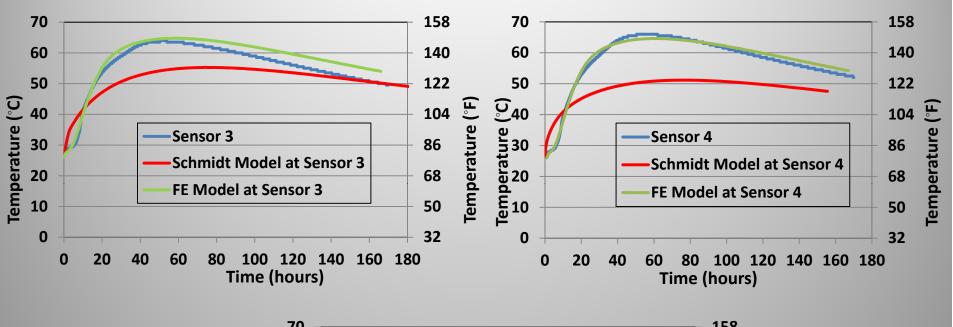
Modeling

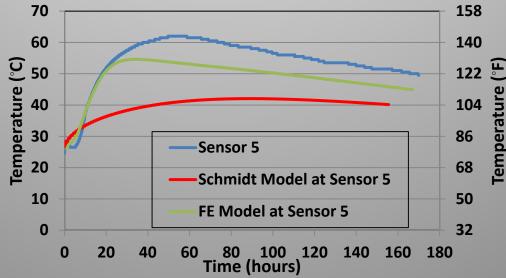
Predicted temperature distribution 167 hours (7 days) after concrete placement

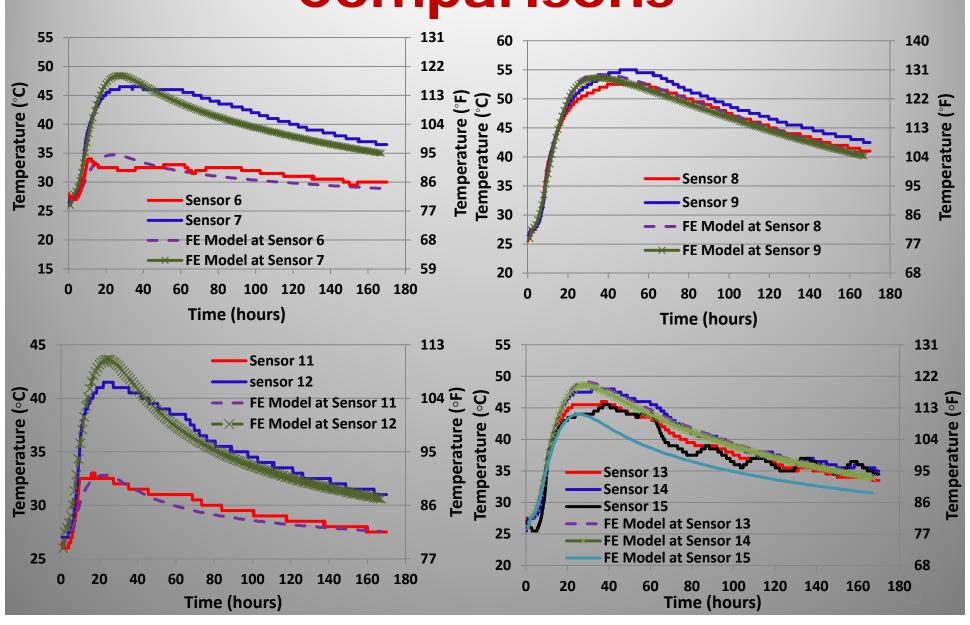


Footing, Miami, FL









Orlando, Florida

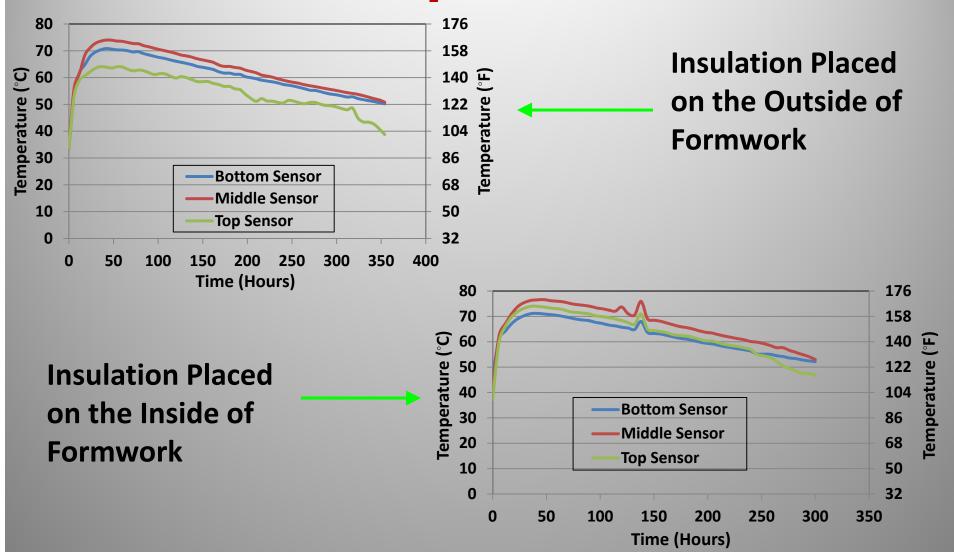


Temperature sensor wires

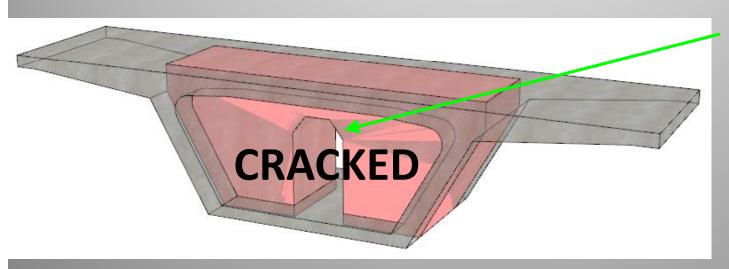
- Concrete Mix Design:
 - Portland Cement Type I/II
 - Fly Ash Class F
 - Ultra-Fine Fly Ash*

^{*-} Used for Strength Only. Not as a requirement for Durability.

- Two methods of insulating the elements were employed:
 - Placing 2" thick Styrofoam (R-5/inch) on the outside of the plywood formwork
 - Placing 2" thick Styrofoam (R-5/inch) on the inside of the plywood formwork

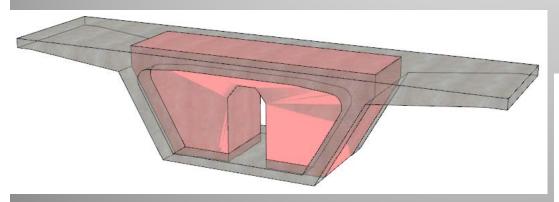


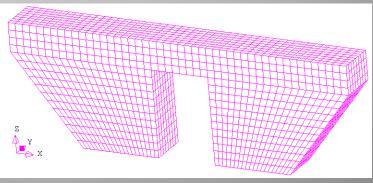
- High-strength concrete: 8500 psi
- Volume to Surface Area ratio: 0.97 ft.
- Was not deemed Mass Concrete



core of the segment was used to calculate the volume to surface area ratio

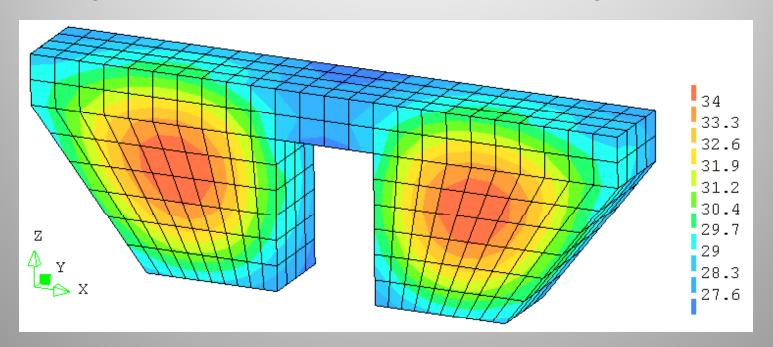
- Should the V/A threshold of 1.0 be reduced when high strength mixes are used?
- What should that threshold be?





- Core length: 18 ft
- Core height: 6 ft
- Two core thicknesses were considered:
 - $-5 \text{ ft} \rightarrow \text{volume to surface area ratio} = 0.97 \text{ ft}$
 - $-4 \text{ ft} \rightarrow \text{volume to surface area ratio} = 0.89 \text{ ft}$
- Hydration data from a normal strength concrete mix design used.

Temperature distribution at 7 days



V/A 0.97: maximum temperature differential = 28.4 °C (51 °F)

V/A 0.89: maximum temperature differential = 26.2 °C (47 °F)

Summary of Findings

 At locations with a high water table (above the footer bottom) soil should not be relied on as a good insulator, it is therefore highly recommended to place a layer of insulating material between the concrete and the soil.

 Placing insulation on the inside of the formwork improves the performance of mass concrete placements in terms of maintaining low delta temperatures.

Summary of Findings

- Preliminary data suggests that the volume to surface area requirements should be reduced when high strength concrete is used for segmental bridge pier structures.
- Structures with least dimension of 6 ft and smaller, concrete with up 48% of the Portland cement replaced with Class F Fly Ash, insulation with R-values 2.5 or greater, consistently maintained delta temperatures below the 35 °F.

Thanks- Any Questions?