

# Effect of Proximity of Sheet Pile Walls on the Apparent Capacity of Driven Displacement Piles (BDV31 TWO 977-26)

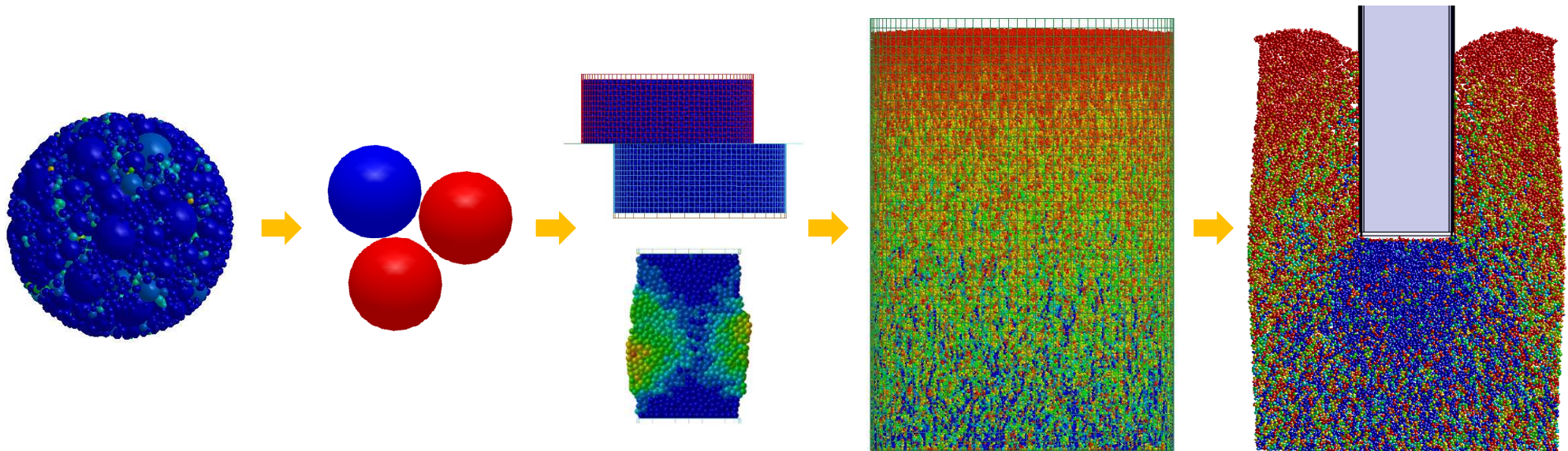
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<sup>3</sup> Geosystems Engineering, ESSIE, University of Florida, Gainesville, FL, USA

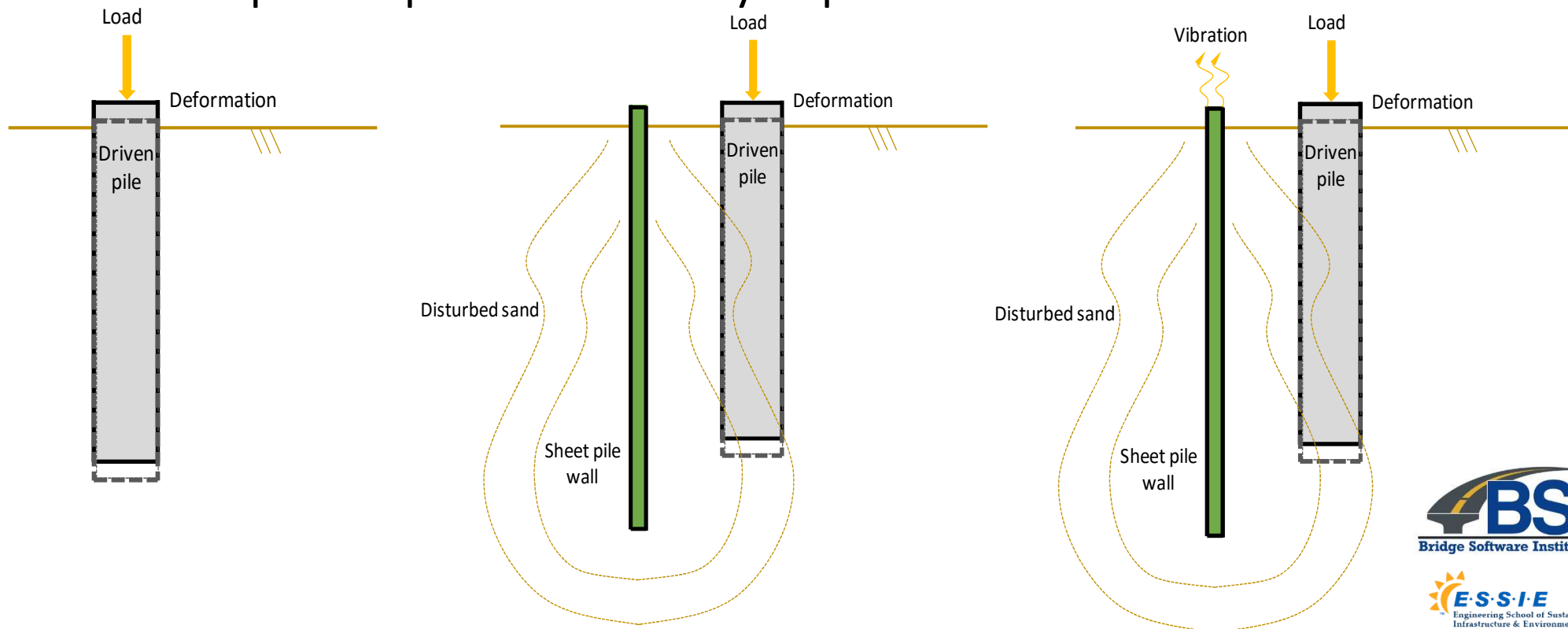


2017 GRIP Meeting  
Gainesville, Florida  
August 18, 2017

# Modeling of Driven Pile and SPW in Granular Materials

- **Deliverables**

- Development of a driven pile model
- Development of a pile and pre-driven SPW model (no removal)
- Development of a pile and pre-driven SPW model (w/ removal)
- Report of parametric study of pile-soil-SPW simulations



# Agenda

- Progress in Geotechnical Centrifuge Modeling
- Progress in Numerical Modeling (in association with centrifuge modeling)

# Geotechnical Centrifuge Modeling

Shear strength and deformation behaviors of granular soils depend on loading history and corresponding granular structure; the main benefit of centrifuge modeling is to simulate repeatable geostatic stress states similar to in-situ conditions.

Quantity	Symbol	Scale Factor
Gravity	$g$	$N$
Length	$l$	$N^{-1}$
Force	$F$	$N^{-2}$
Stress	$\sigma$	$1$
Acceleration	$a$	$N$
Time (Dynamic)	$t_{\text{dyn}}$	$N^{-1}$
Time (Diffusion)	$t_{\text{diff}}$	$N^{-2}$
Frequency	$f$	$N$

# UF Centrifuge Equipment



## *UF Centrifuge*

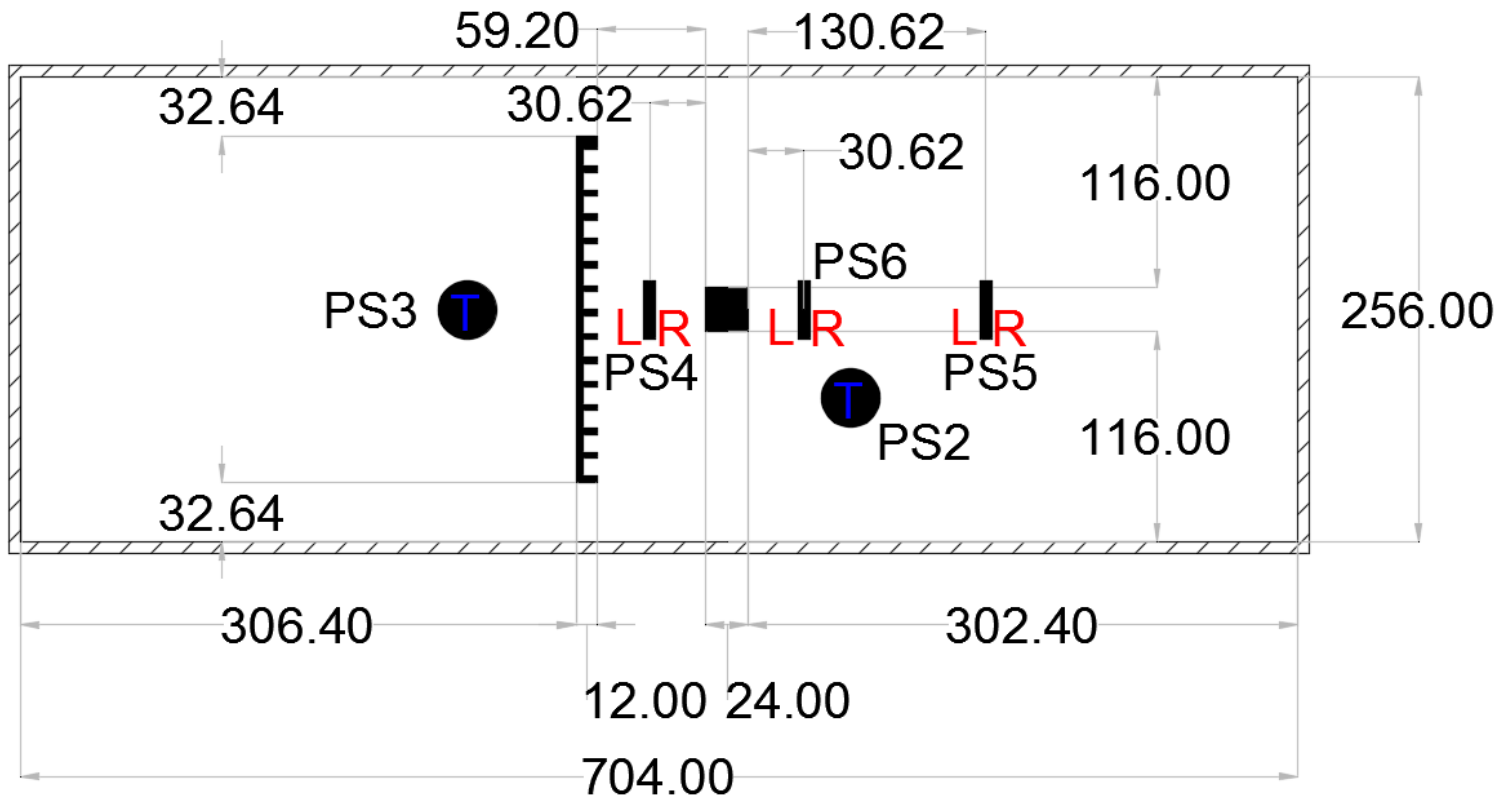
*Radius = 1.5 m*

*Max. Payload = 12.5 g-ton*

*Max. Acceleration = 80 g*

*Centrifugal Acceleration =  
32 g*

# Task 4. Centrifuge Test Set-up: Plan View

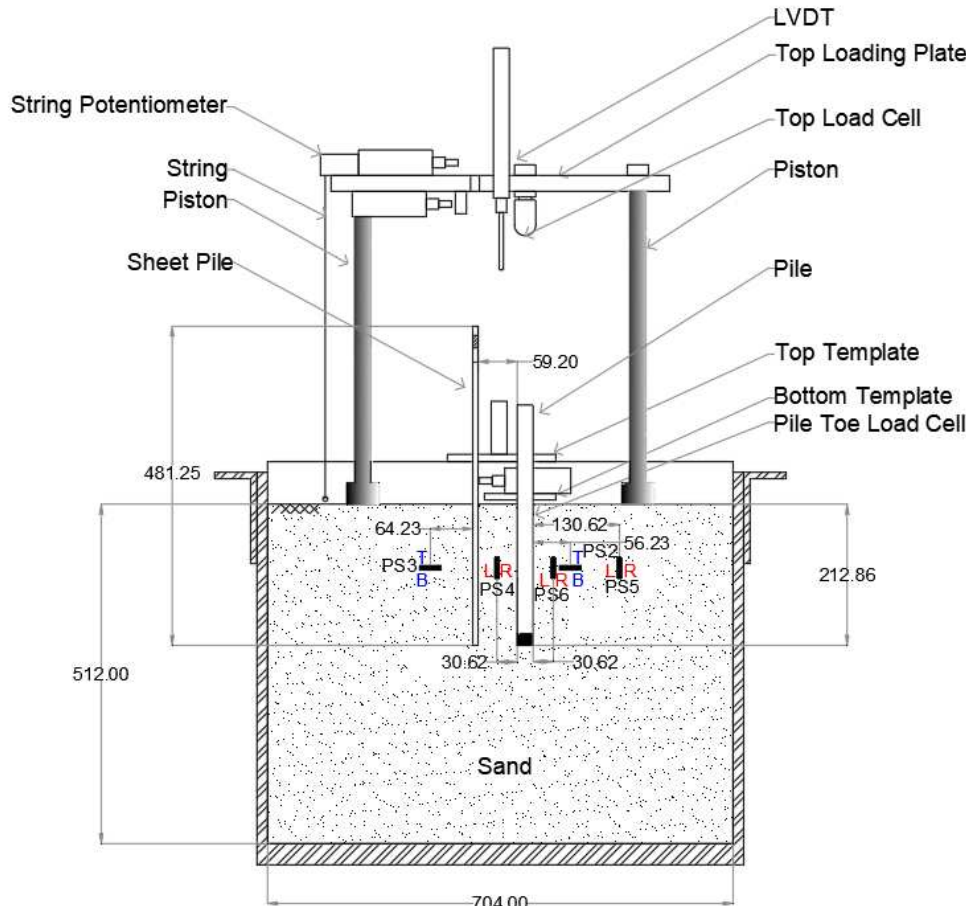
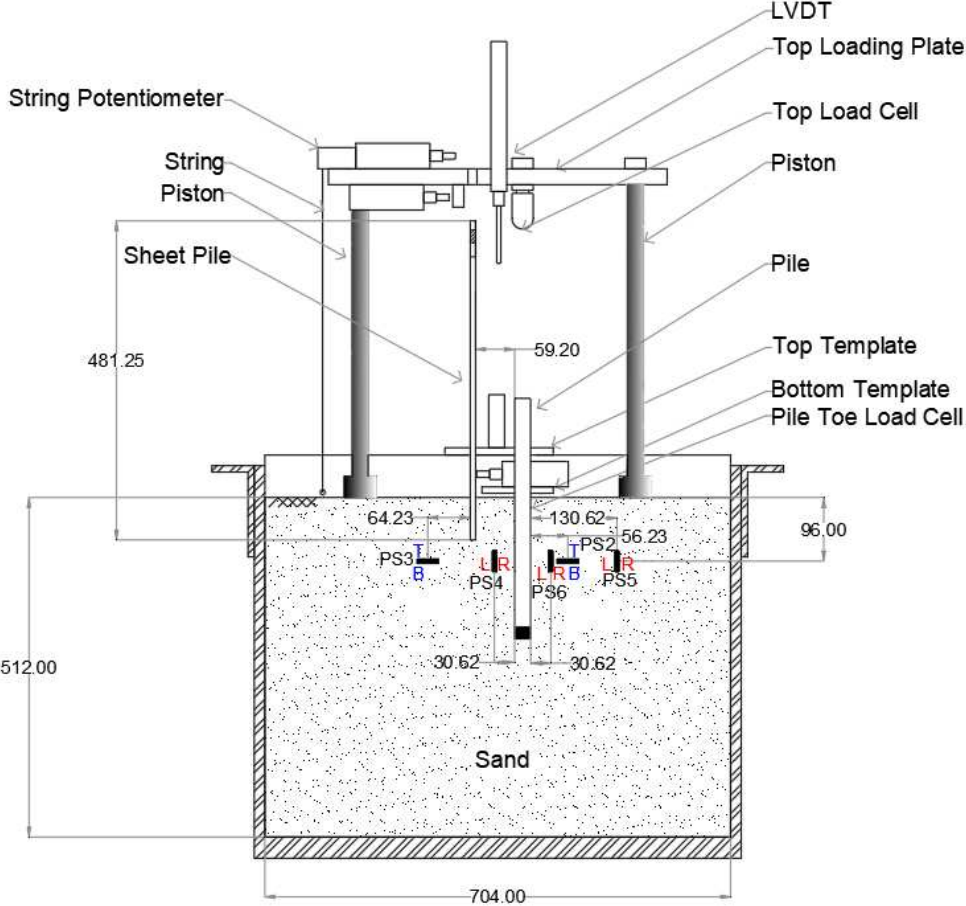


Prototype scale dimensions (inches)





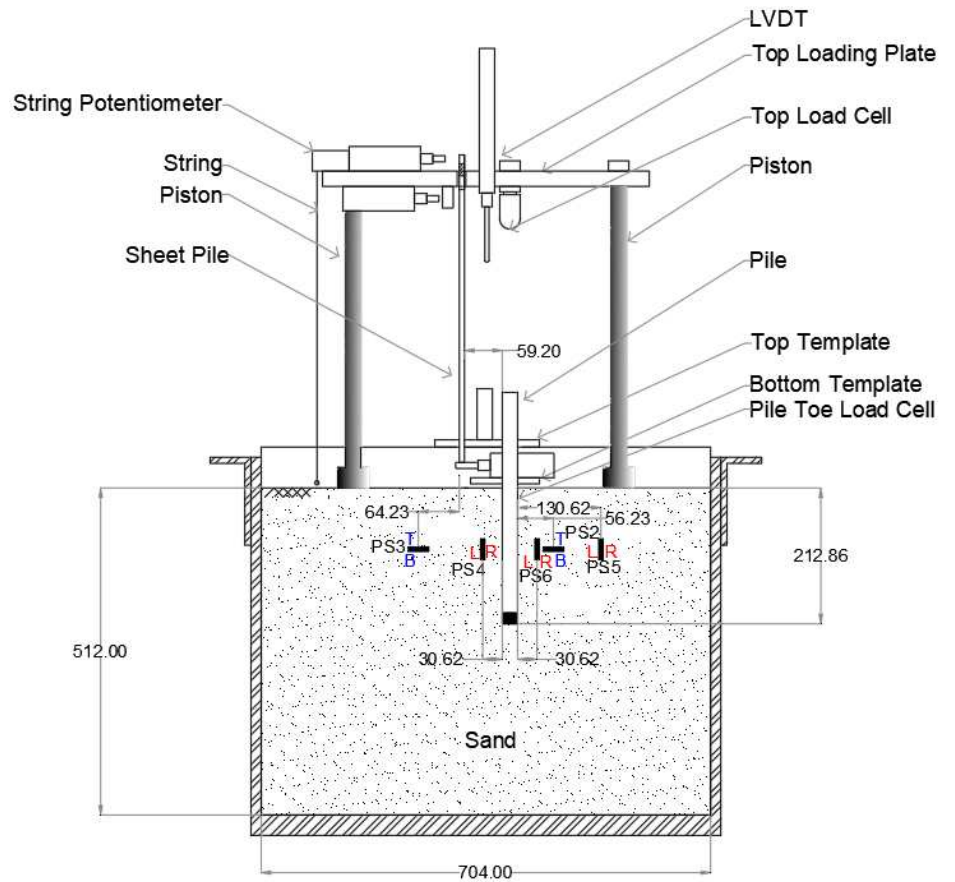
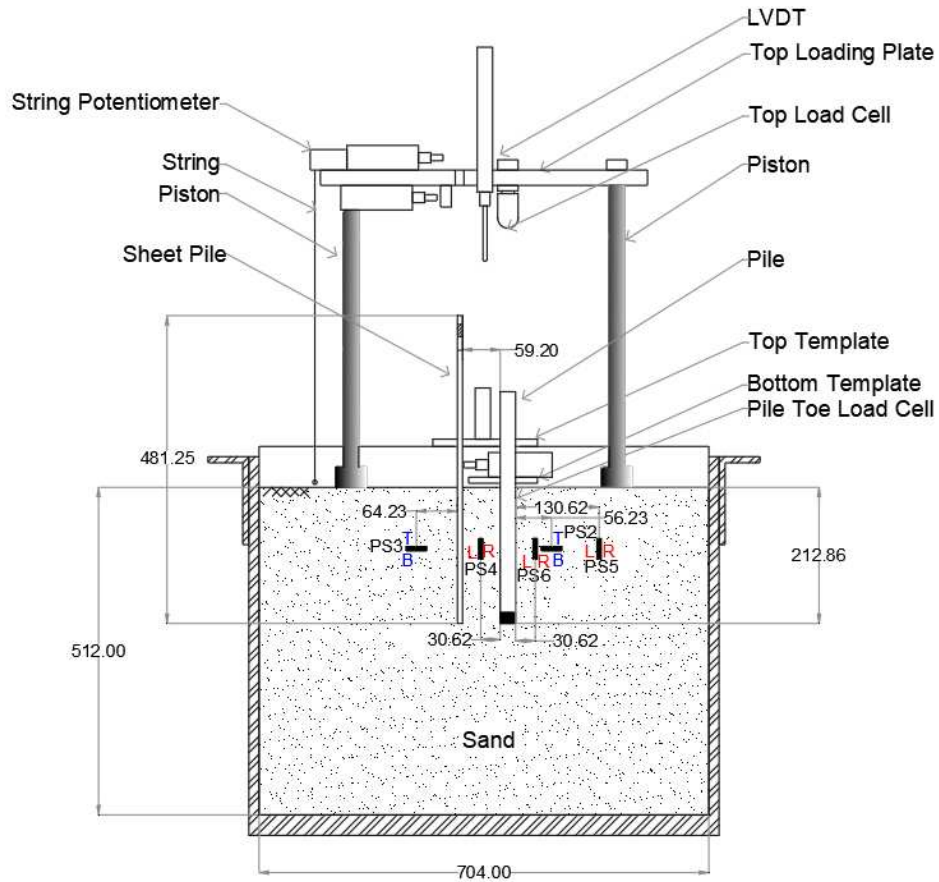
# Centrifuge Test Set-up: Loading Scenario 2



Prototype scale dimensions (inches)



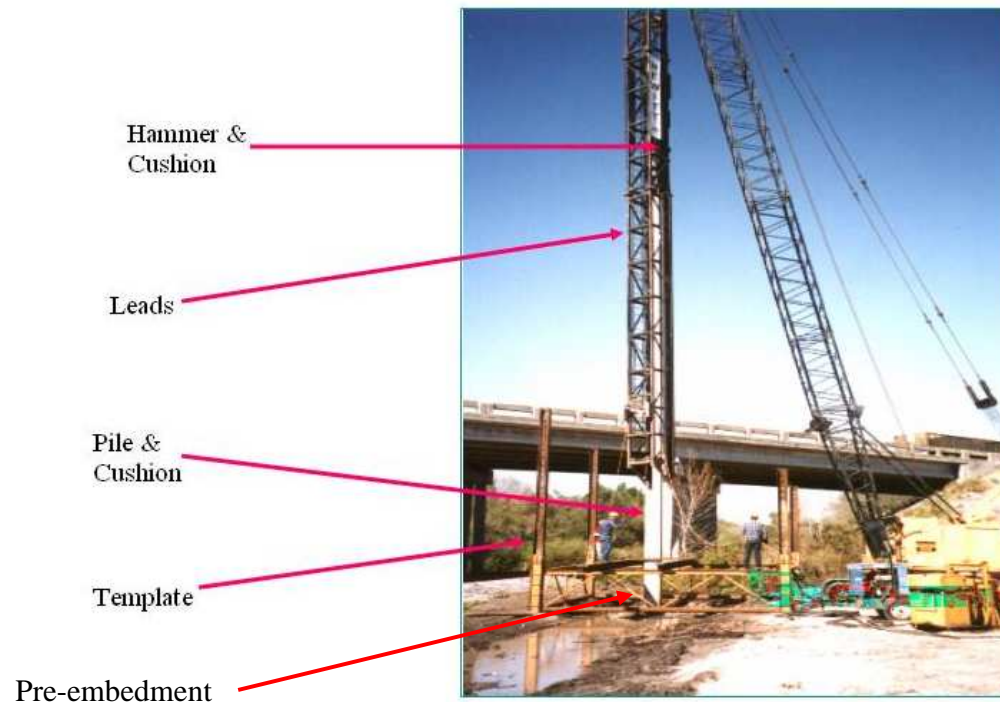
# Centrifuge Test Set-up: Loading Scenario 3



Prototype scale dimensions (inches)

# Dimensions of Prototype Pile

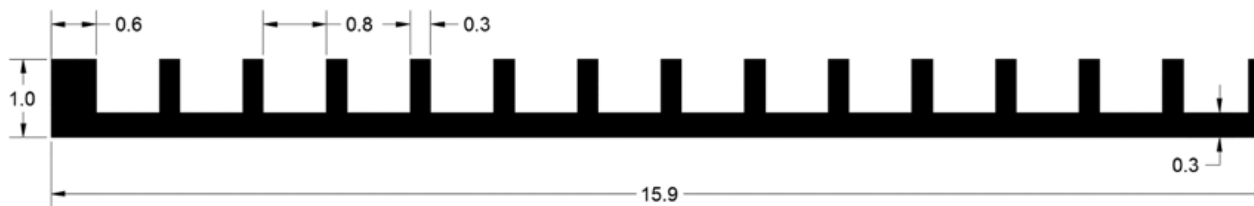
Scale	Scale factor	Length (in)	Outside width (in)	Area (in <sup>2</sup> )	Modulus of Elasticity (ksi)	E.A (kips)	Pre-embedment depth (in)	Total embedment depth (in)
<b>Model</b>	1.0	11.33	0.75	0.31	$1.0 \times 10^4$	3125	0.5	6.65
<b>Prototype</b>	32.0	362.4	24	320	$1.0 \times 10^4$	$3.2 \times 10^6$	16	212.86



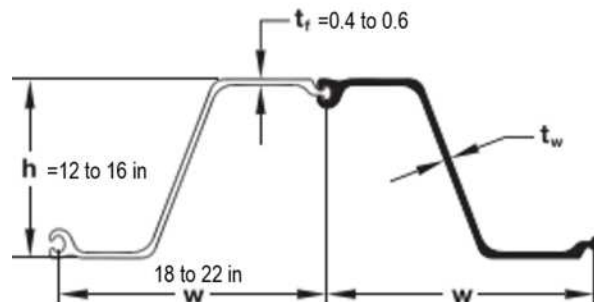
Pile Driving

# Dimensions of Sheet Pile Wall (SPW)

Scale	Scale factor	Length (in)	Width (in)	Wall thickness (in)	Area (in <sup>2</sup> )	Modulus of Elasticity (ksi)	E.A (kips)	Pre-embedment depth (in)	Total embedment depth (in)
<b>Model</b>	1.0	15.04	5.96	0.375	1.14	$2.9 \times 10^4$	33147	2	6.65
<b>Prototype</b>	32.0	481.25	190.72	12	27796	$2.9 \times 10^4$	$8.1 \times 10^8$	64	212.86

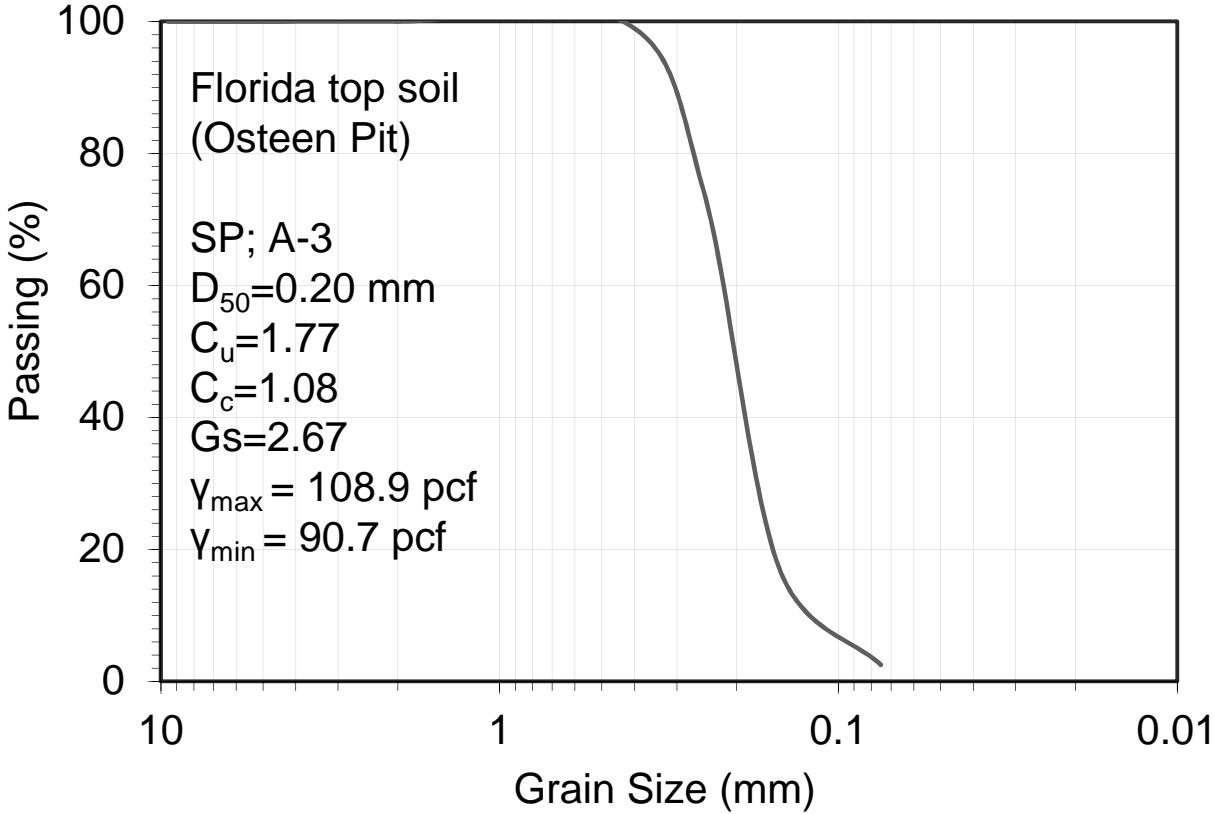


Plan view of the sheet pile wall with tongue-groove pattern in centrifuge tests



Plan view of a typical steel sheet pile wall

# Granular Soil for Centrifuge Model



Laboratory test results provided by SMO



# Preparation of Centrifuge Models



Target relative density of FL sand (by pluviation) =  
60%,  $K_0=0.5$ ,  $\phi=30^\circ$



# Instrumentation in Centrifuge Models

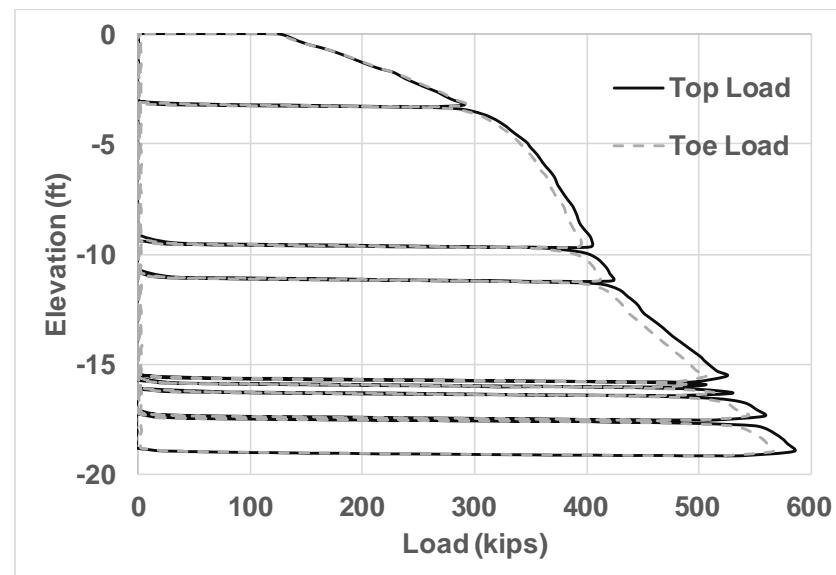
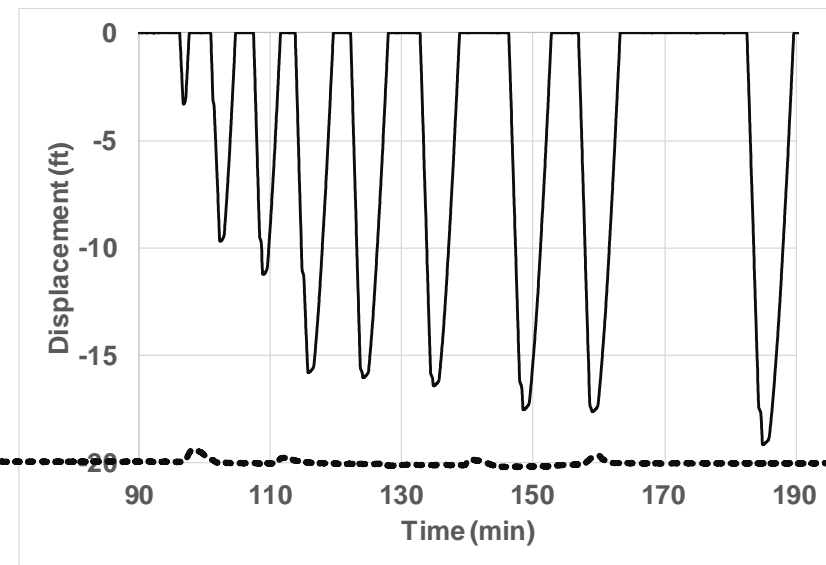
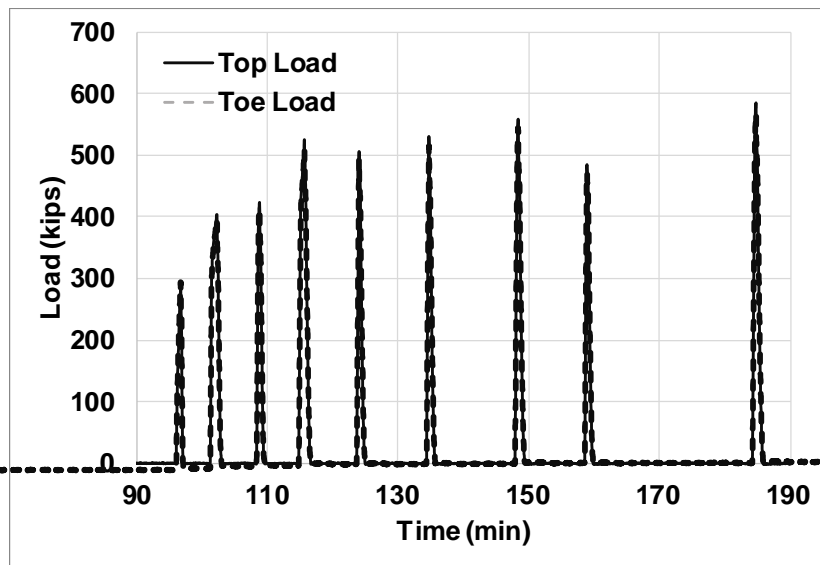


# Centrifuge Apparatus and Load Mechanism



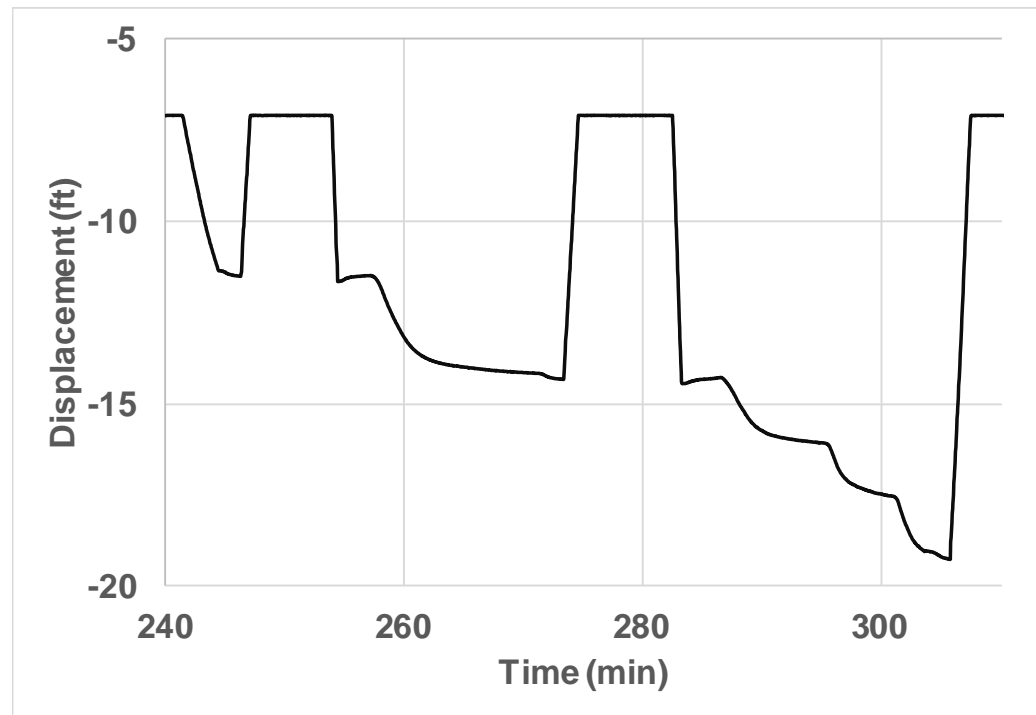
# Displacement and Load-Time Histories during Pile Driving

All values are in prototype scale.



# Displacement-Time History during SPW Driving

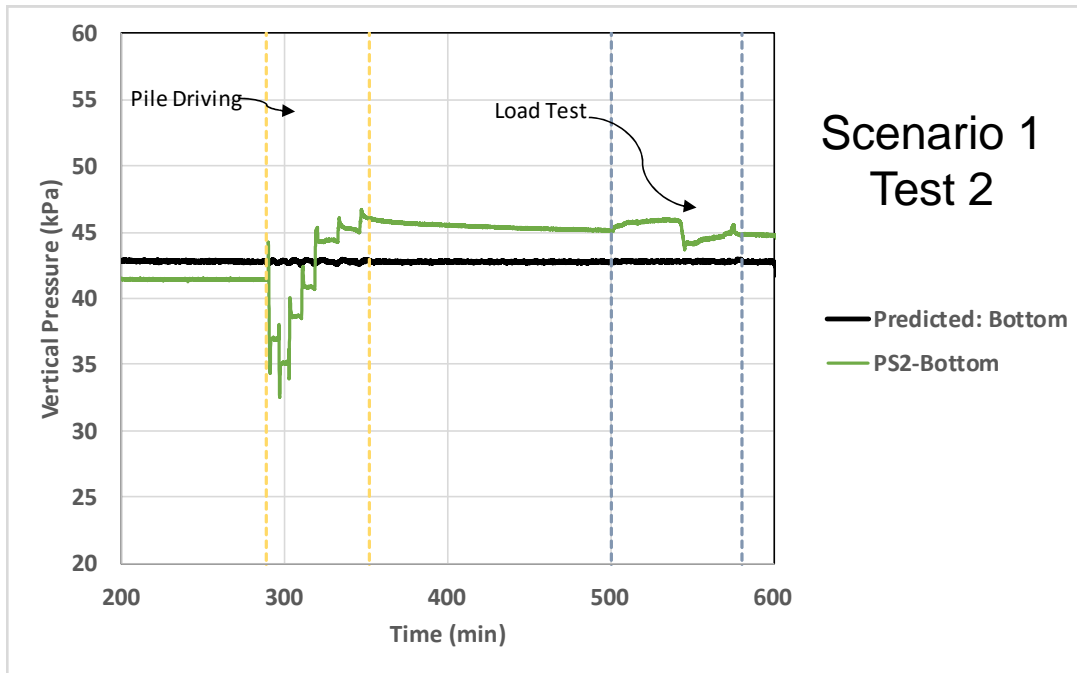
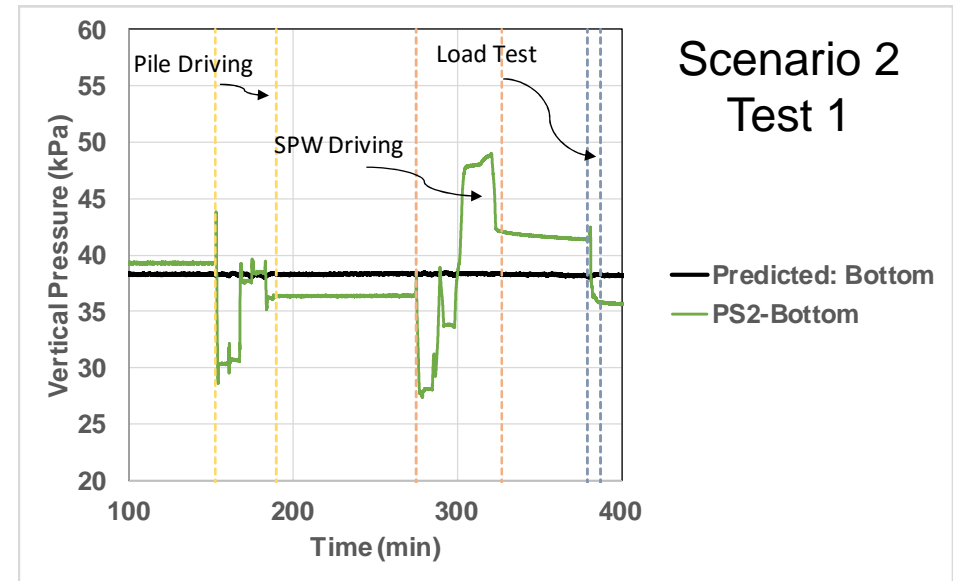
All values are in prototype scale.



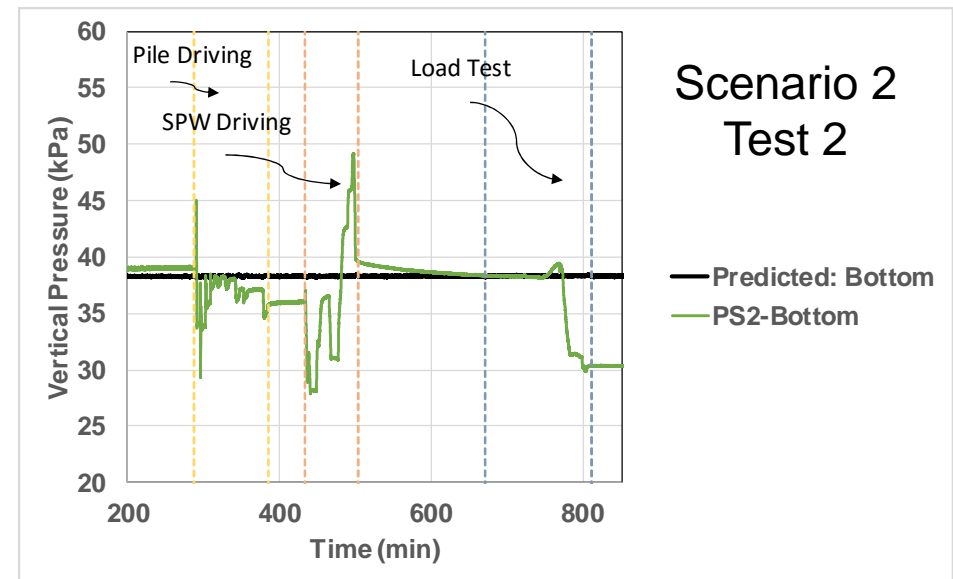


# Vertical Pressure – Time History (at 2.8D Right Side of the Pile)

Pressure sensor depth = 8 ft  
 Pile Driving in 5 increment: -6.2 → -8.4 → -12.7 → ... → -17.8 ft



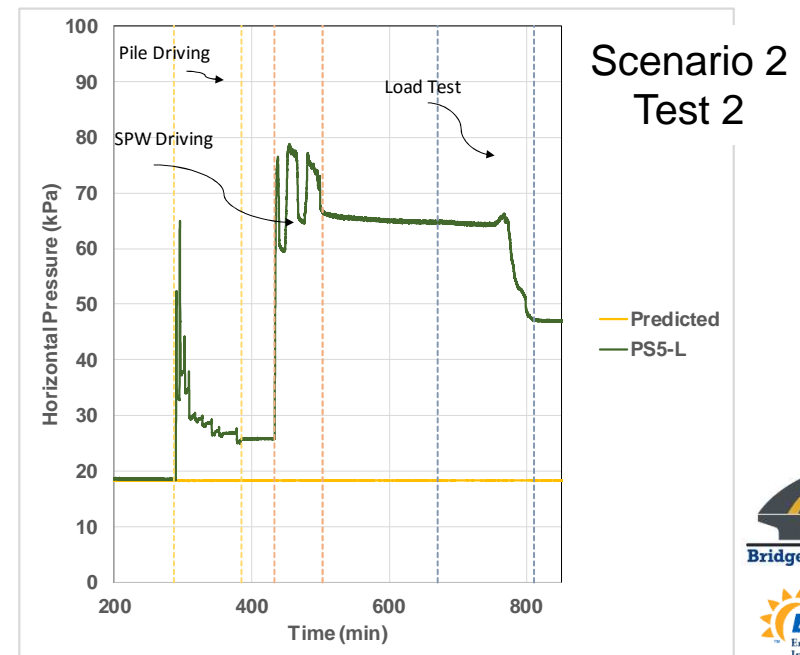
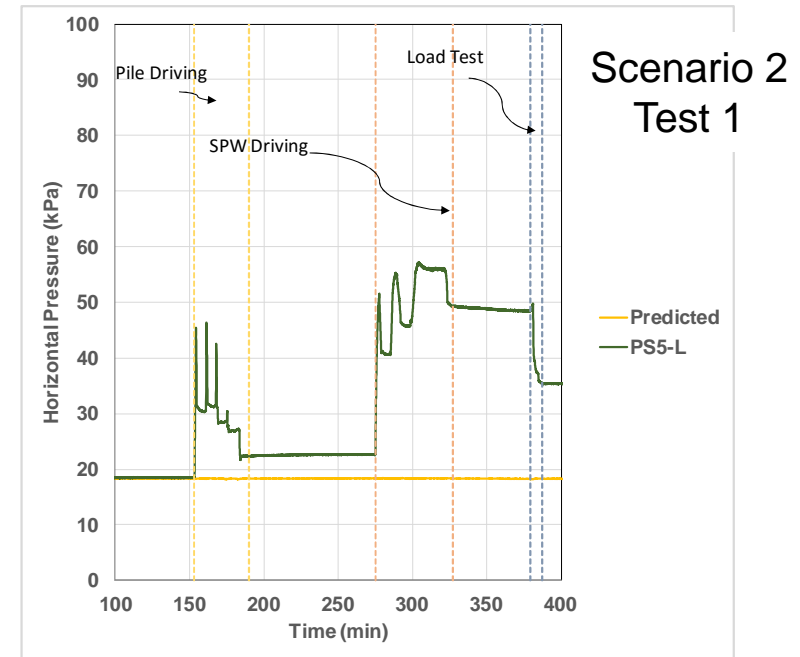
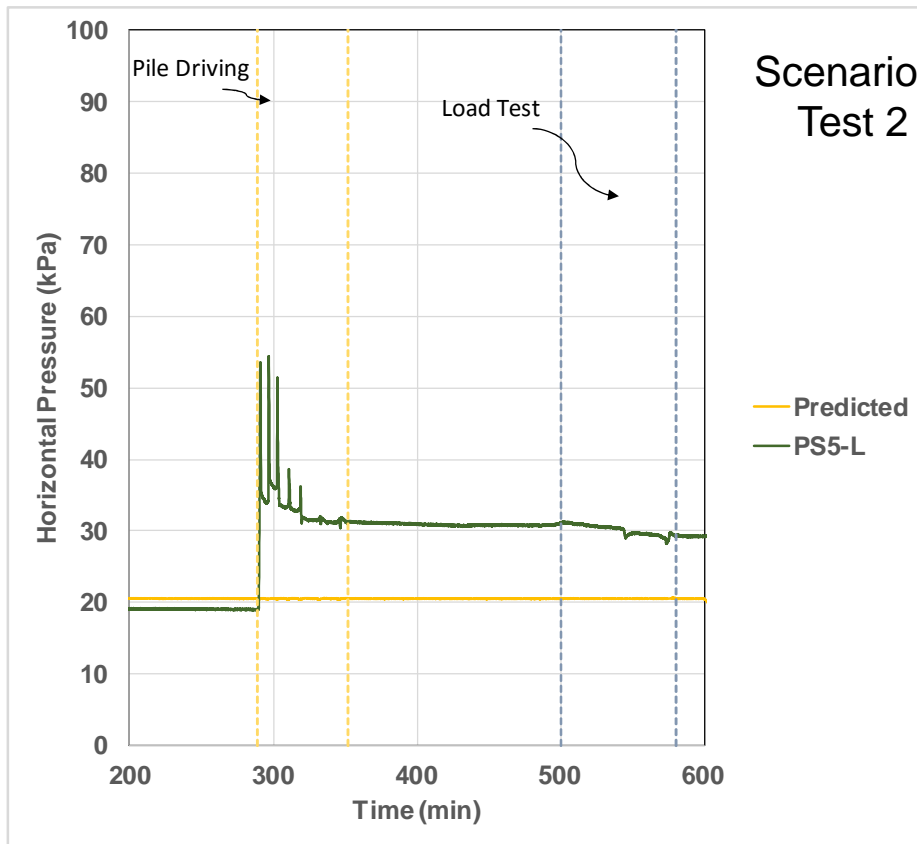
Pressure sensor depth = 8 ft  
 Pile Driving in 7 increment: -6.7 → -8.2 → -12.4 → ... → -17.8 ft



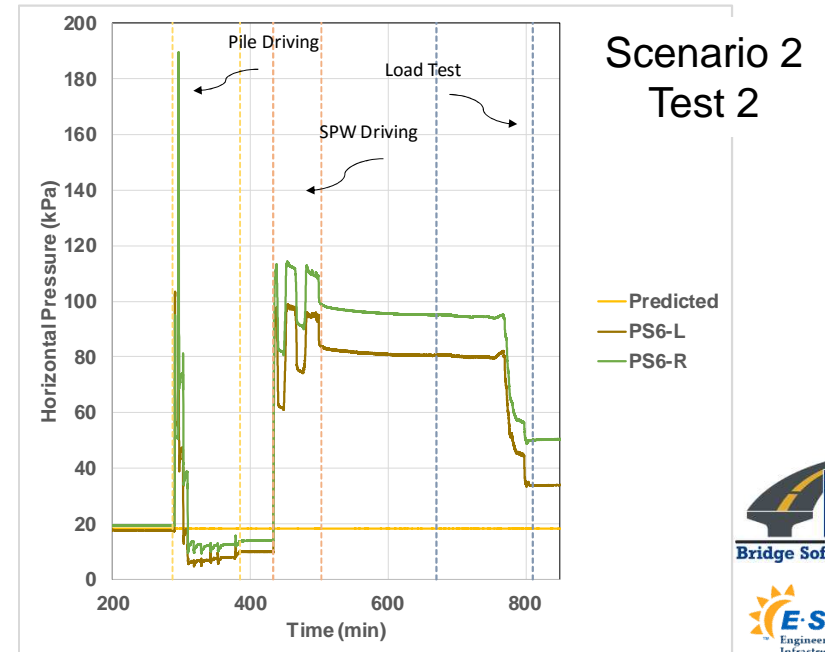
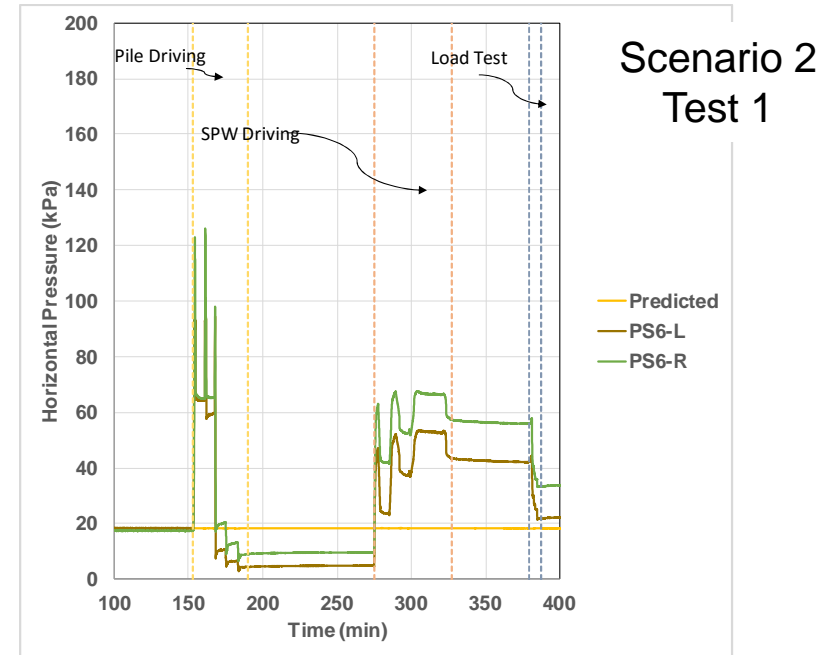
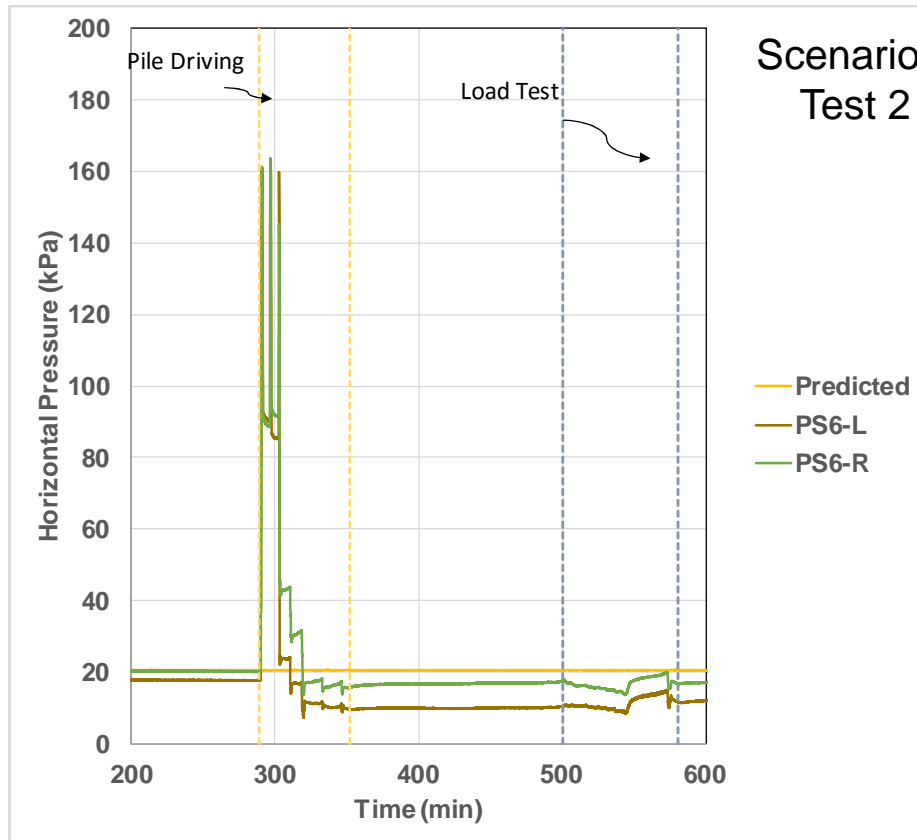
Pressure sensor depth = 8 ft  
 Pile Driving in 9 increments: -3.3 → -9.7 → -11.2 → ... → -17.8 ft



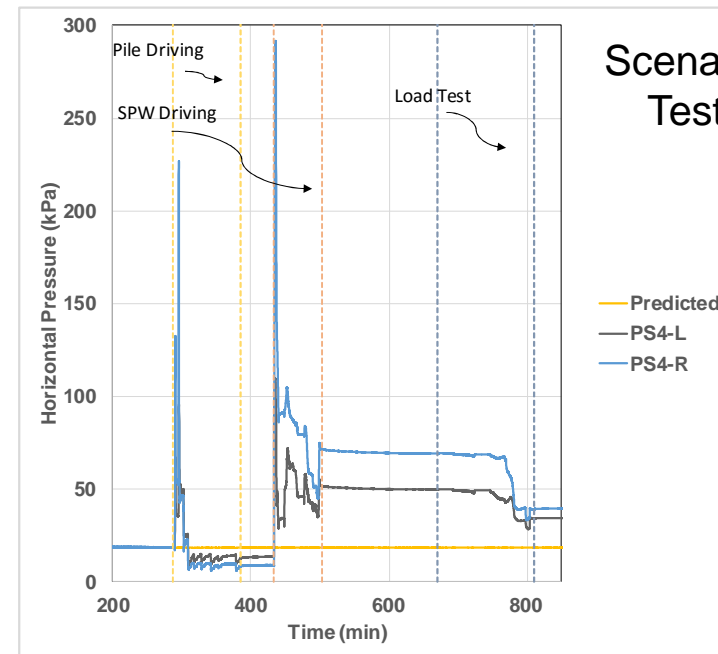
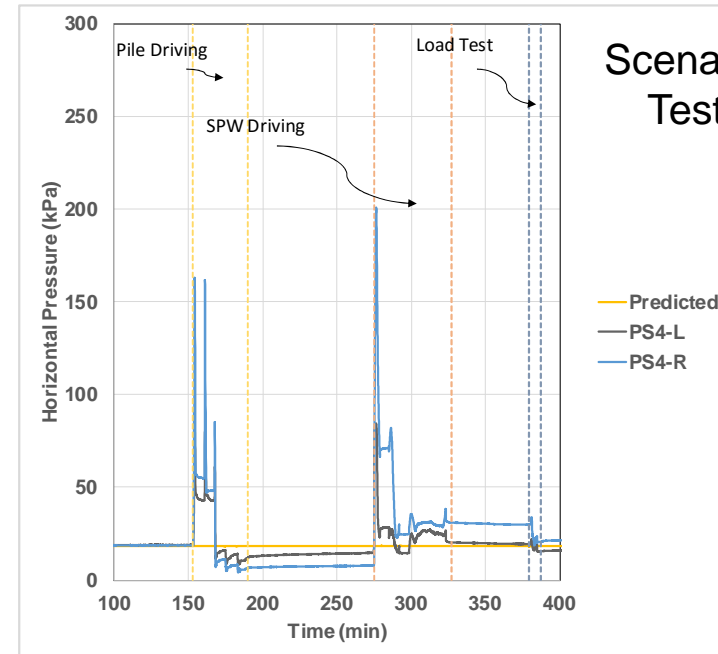
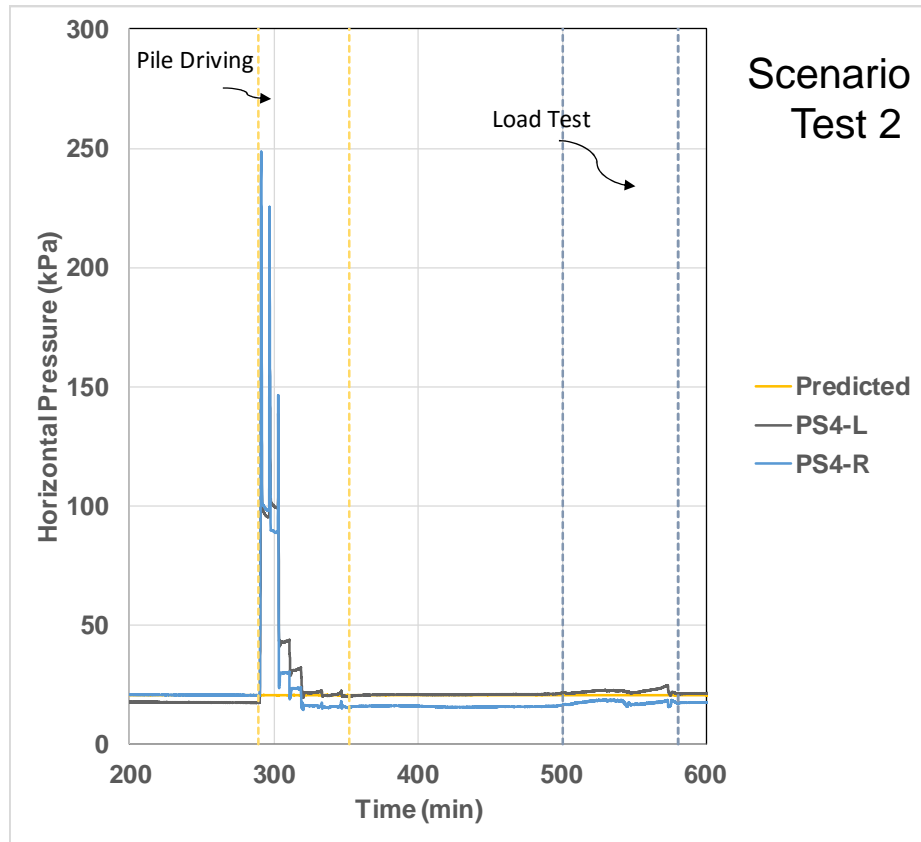
# Far-Field Horizontal Pressure – Time History (at 5.9D Right Side of the Pile)



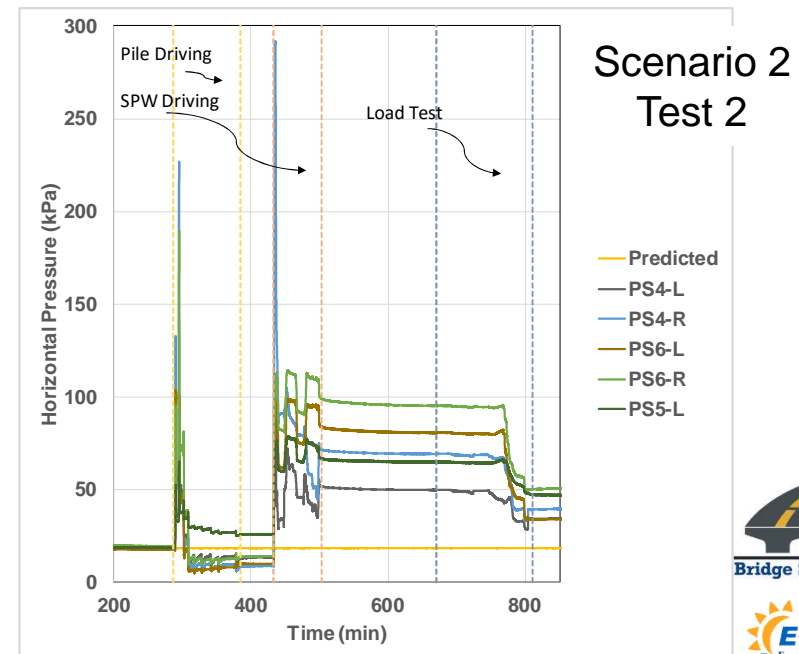
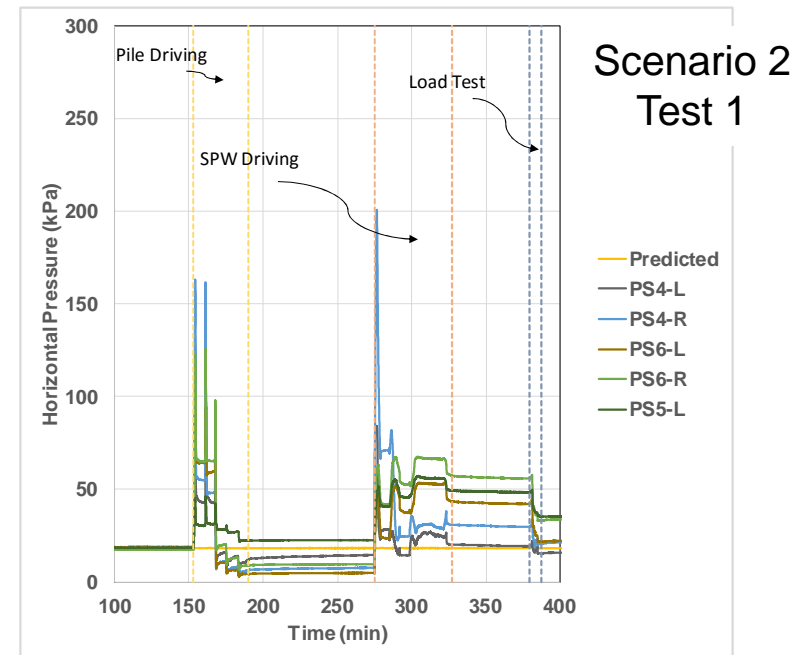
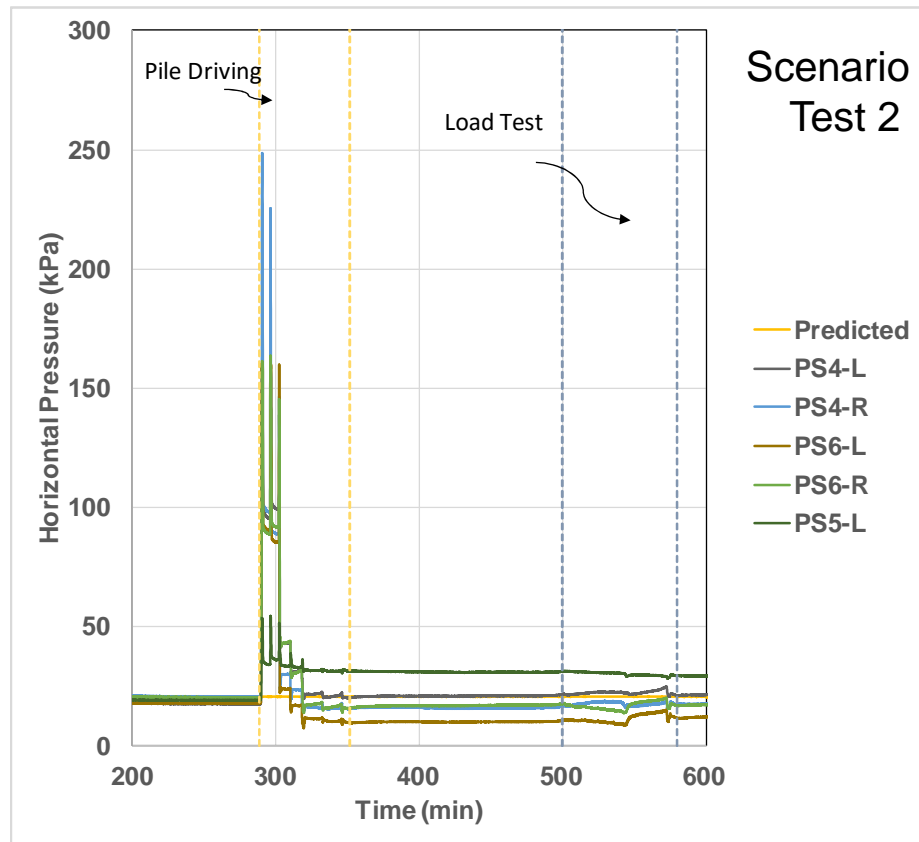
# Near-Field Horizontal Pressure – Time History (at 1.8D Right Side of the Pile)



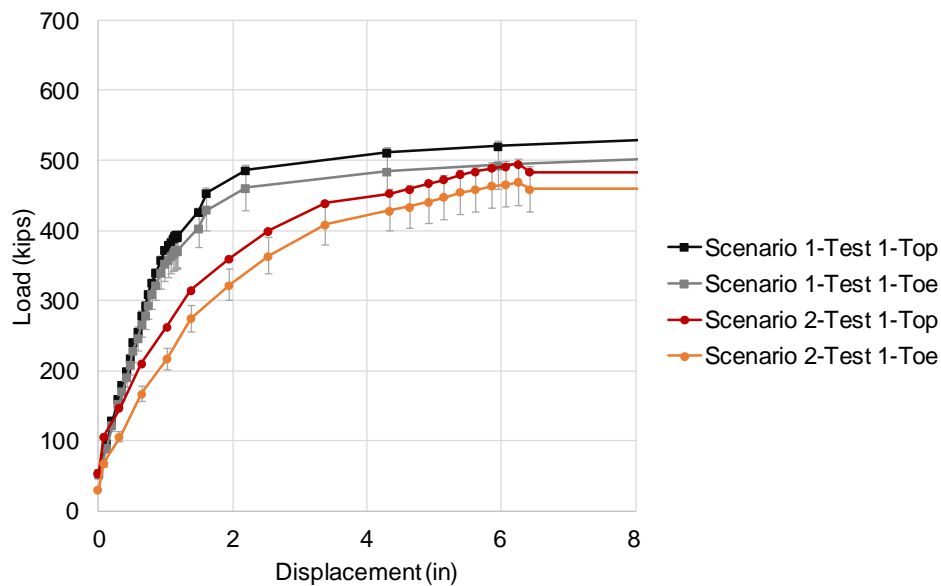
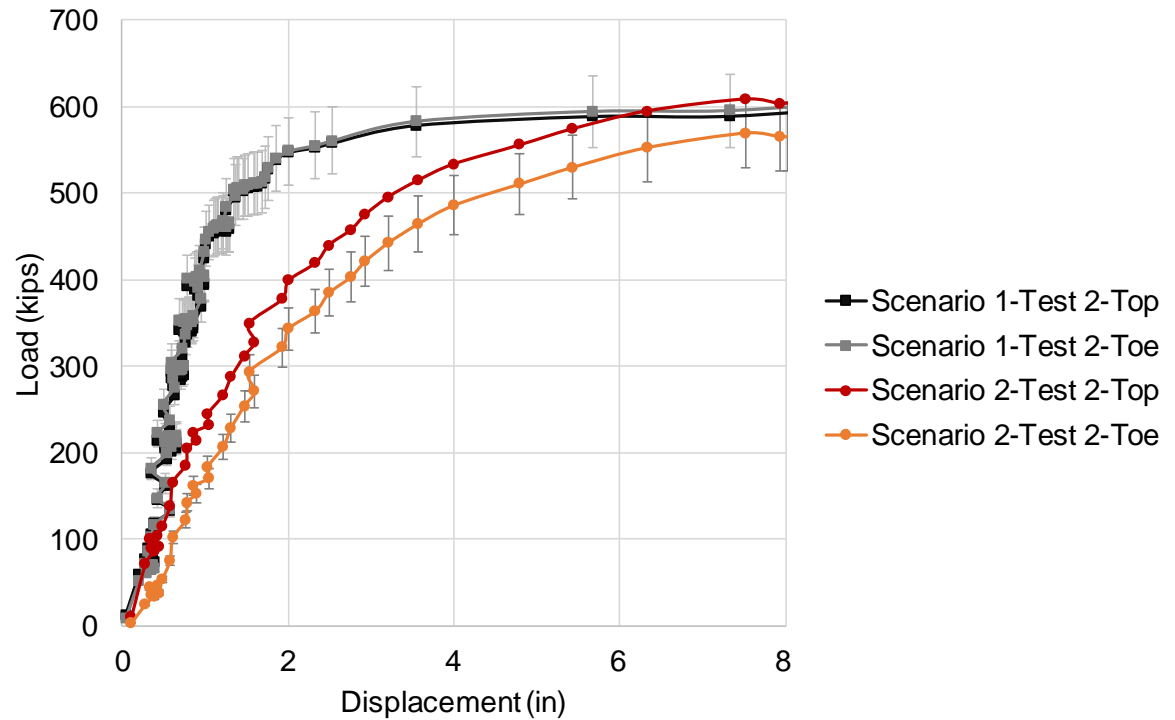
# Horizontal Pressure between Pile and SPW (at 1.8D Left Side of the Pile)



# Comparative Horizontal Pressure – Time History



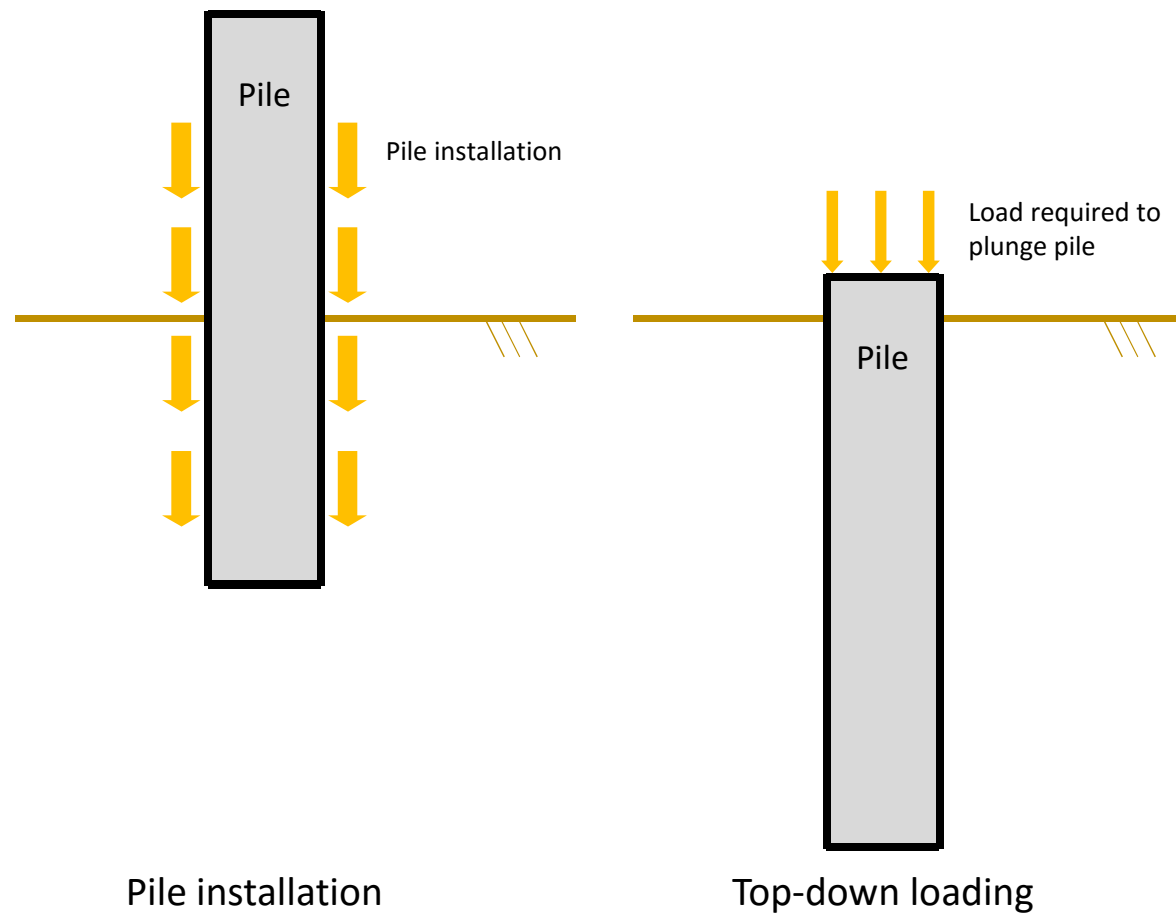
# Centrifuge top-down load tests results





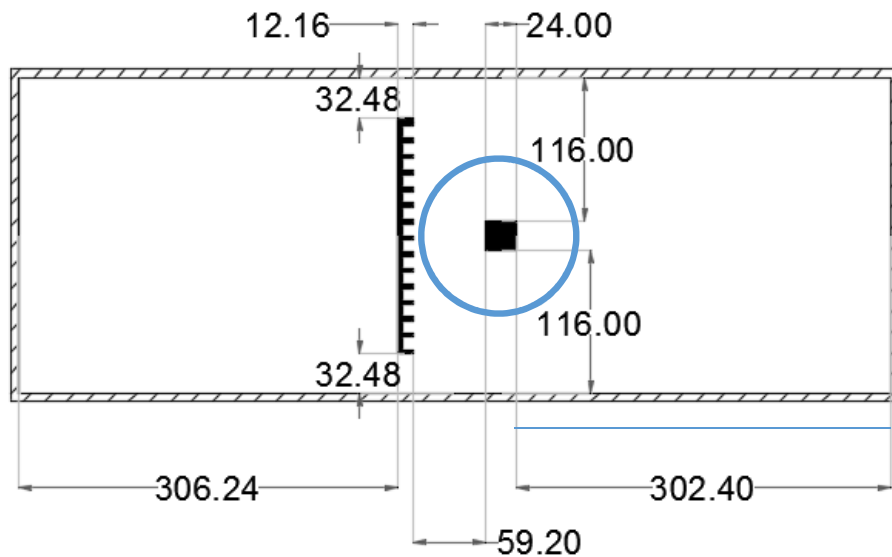
# Task 3. Numerical Modeling of Driven Pile and SPW in Granular Soil

- Scenario 1: Driven pile

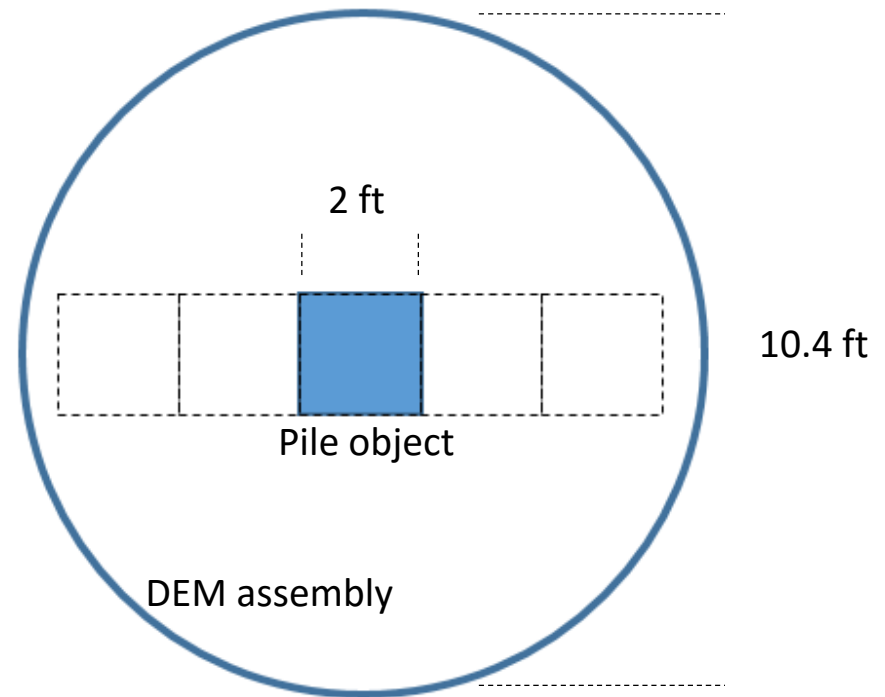


# DEM-FEM Modeling of Scenario 1

- Model overview:
  - Soft-particle dynamics
  - Prototype scale (32g)



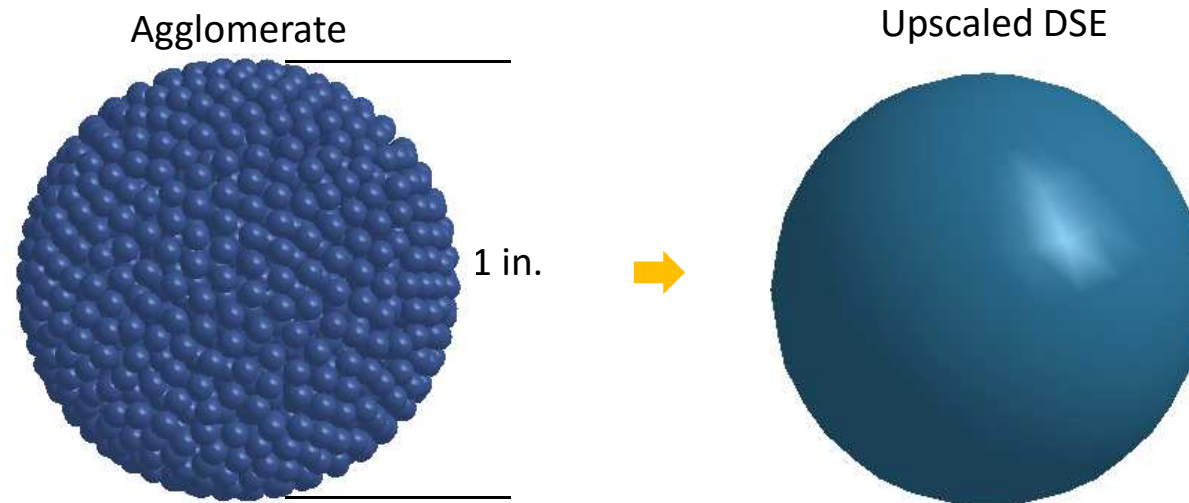
Plan view of centrifuge test setup



Plan view schematic of numerical model

# Task 3. Numerical Modeling

- Mesoscale Discrete Spherical Element (MDSE)
  - Modeling DSE at grain scales is not feasible for prototype-scale domain
    - Requires  $1E+09 \sim 1E+10$  elements
    - Current limits are  $\sim 1E+07$  elements
  - Averaged values of contact stiffness and friction coeff. are mapped onto the representation of upscaled DSE
  - Partitioning of continuum-scale volume (e.g., a 4x8 in. cylinder) requires mass-averaging to ensure conservation of mass as per weight and void ratio of a sample.



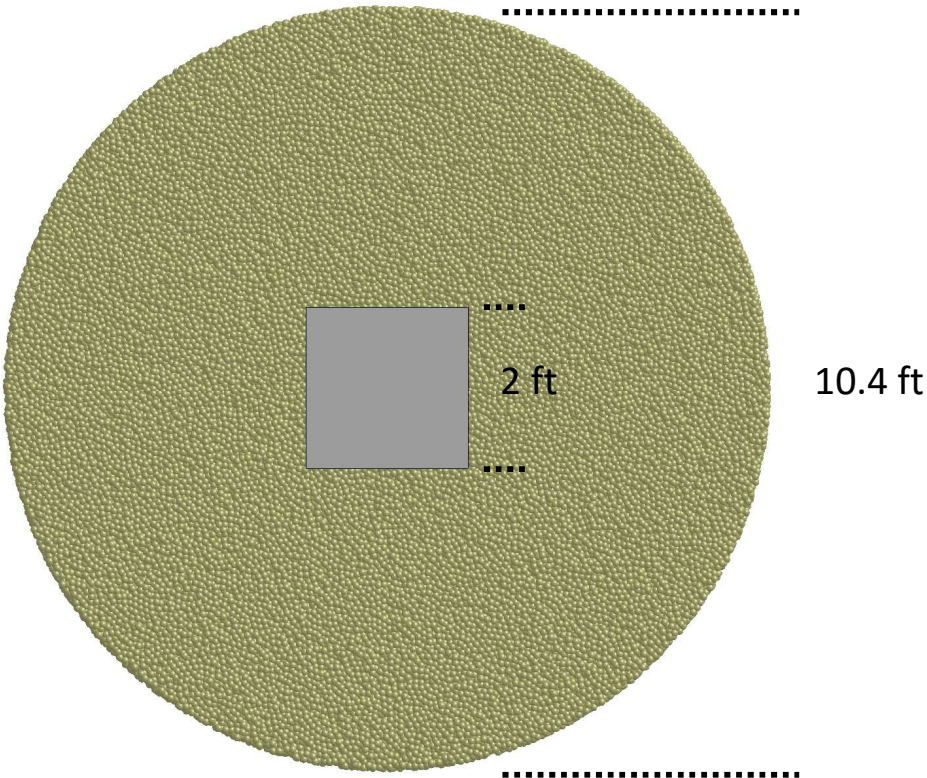
# Parameters of Upscaled DSE

- Parameters of mesoscale DSE for use in pile-driving and top down load test simulations:
  - Mass density of upscaled DSE based on void ratios of medium dense sand ( $D_r = 58\%$ ;  $D_{50} = 0.32$  mm;  $\gamma = 102$  pcf; Yamamuro et al. 2011)
  - Model prediction indicates that the extended DEM is applicable to simulation of coarse-grained soil with  $\phi = 30^\circ - 32^\circ$
  - Inter-granular sliding and rolling friction coefficients are parameters to simulate various shear strengths of macro-volumes.
  - Low restitution: constituent particle coulombic damping is prevalent.

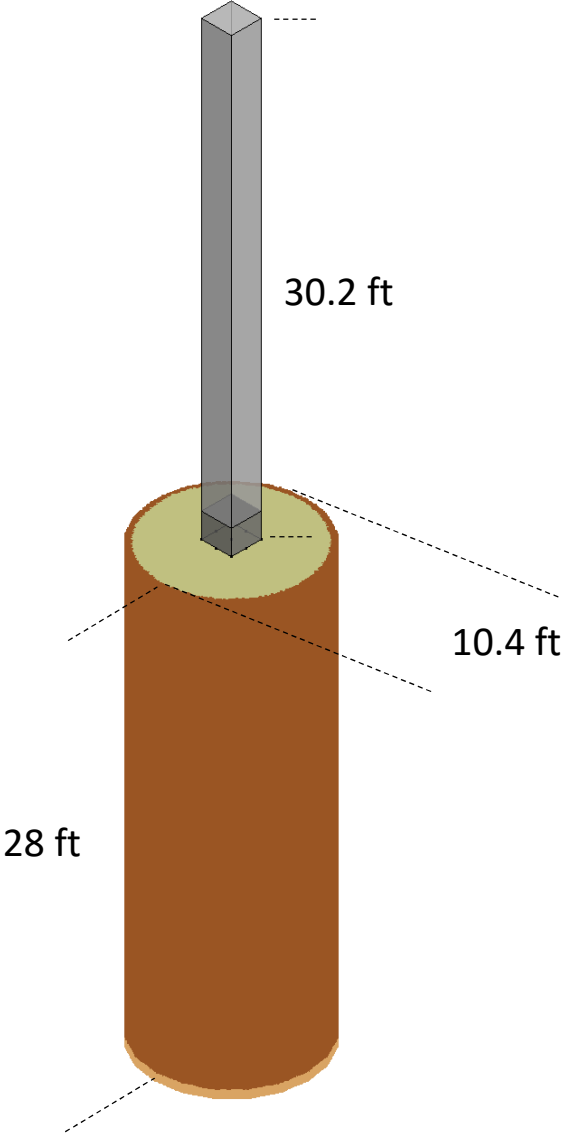
Property	Value	Unit
Diameter	1.0	in
Density (by weight)	101	lb/ft <sup>3</sup>
Bulk modulus	24	ksi
Inter-granular friction coefficient	0.47	--
Coefficient of restitution	0.001	--

# DEM-FEM Modeling of Scenario 1

- Model overview:
  - Soft-particle dynamics
  - Bulk unit weight: 101 pcf



Plan view of aluminum pile



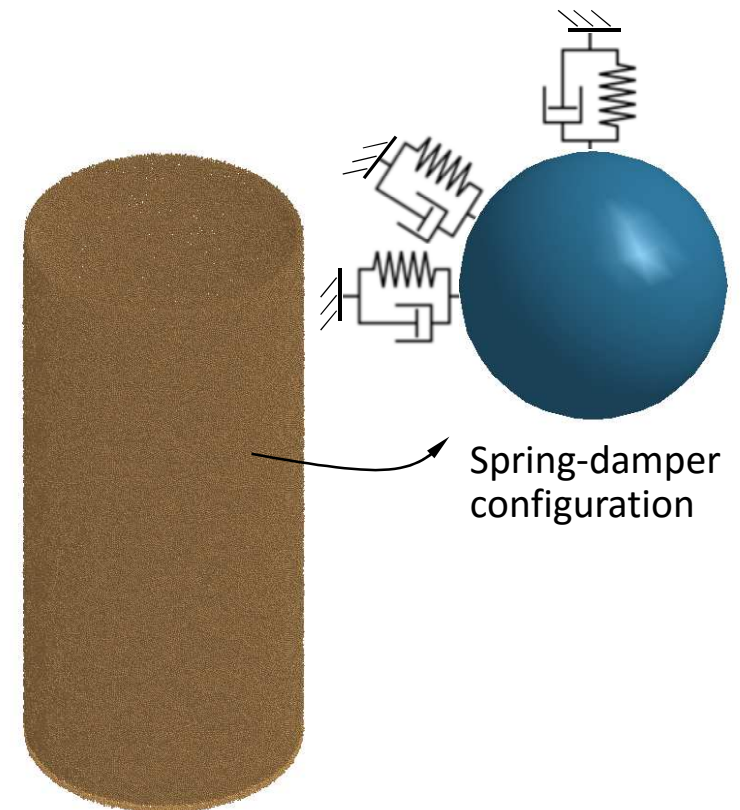
DEM-FEM model

Source for pile and sheet pile wall prototype object dimensions: Preliminary Task 3/Task 4 report, submitted Dec. 2016



# DEM-FEM Modeling of Scenario 1

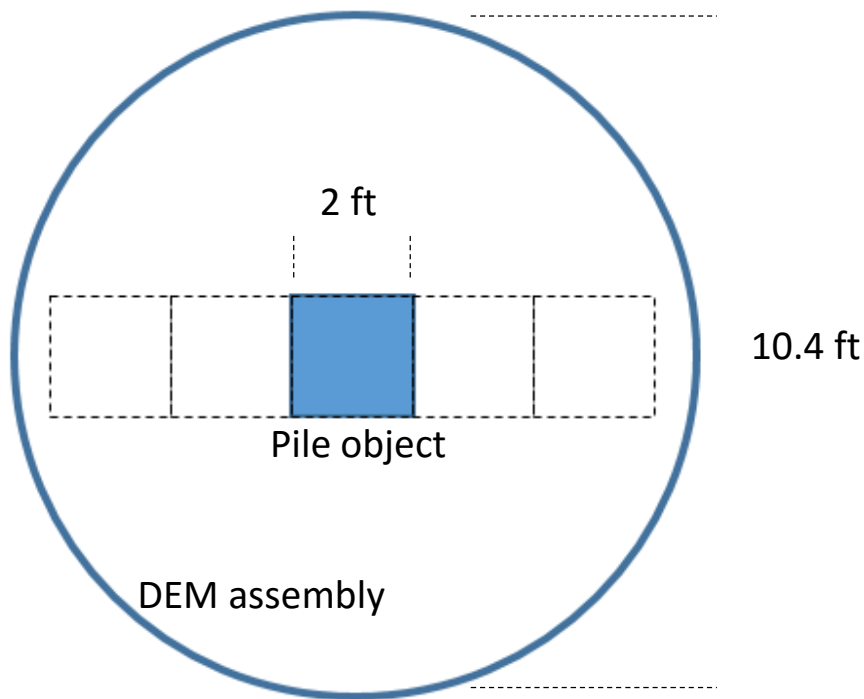
- Boundary conditions:
  - Network of boundary spheres
    - Sides
    - Bottom
  - Confining Pressure
    - Maintains horizontal/vertical stresses
    - Incorporated as spring initial offsets
  - Spring-damper
    - Each translation DOF
    - Spring stiffness tributary to confinement
    - Dampers match sphere-sphere contact



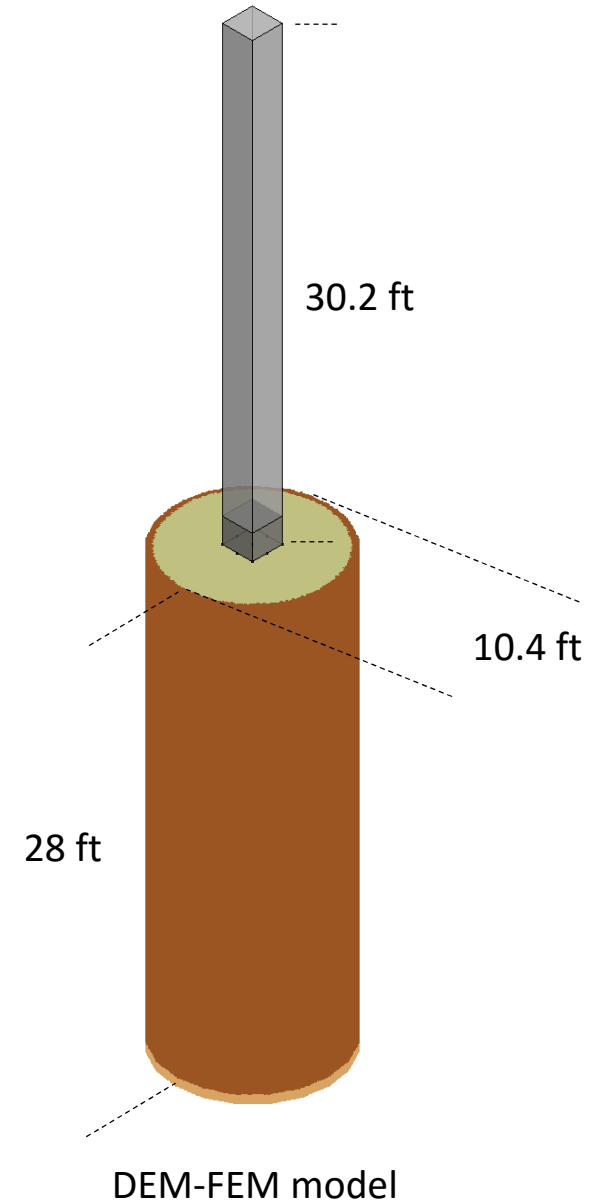
Boundary condition modeling

# DEM-FEM Modeling of Scenario 1

- Model overview:
  - Soft-particle dynamics
  - Cylindrical configuration
    - ~4.8 million spheres (1 in. diameter)
    - Est. 7 days simulation



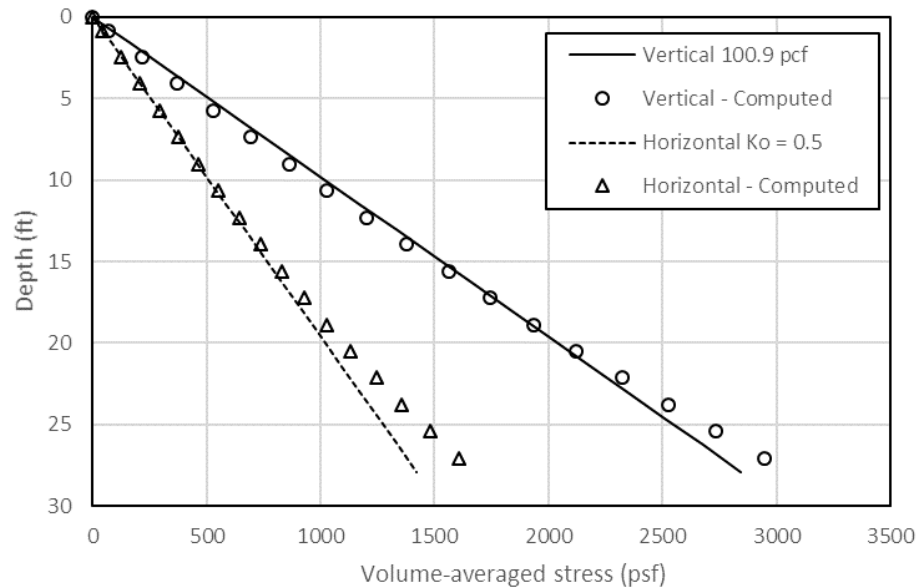
Plan view schematic of numerical model



DEM-FEM model

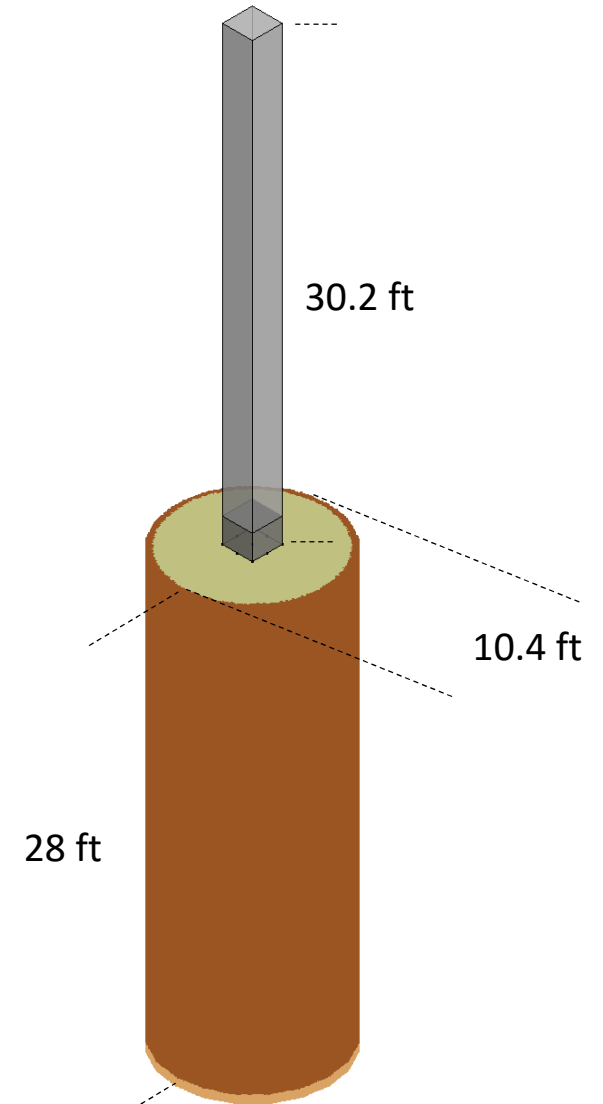
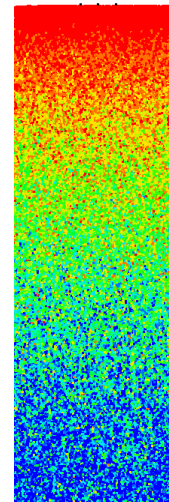
# DEM-FEM Modeling of Scenario 1

- Simulation stage 1:
  - Gravity acting on DEM assembly
  - Geostatic stresses consistent with physical tests
  - $K_o = 0.5 \rightarrow \text{Phi} \sim 30^\circ$



Geostatic stresses for DEM assembly under gravity

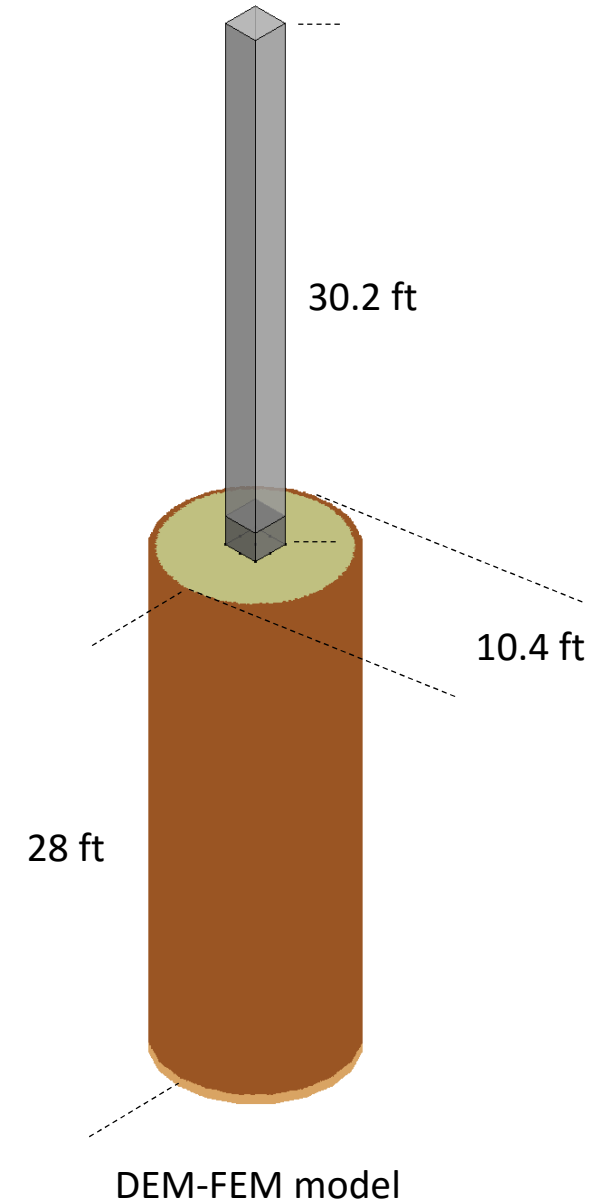
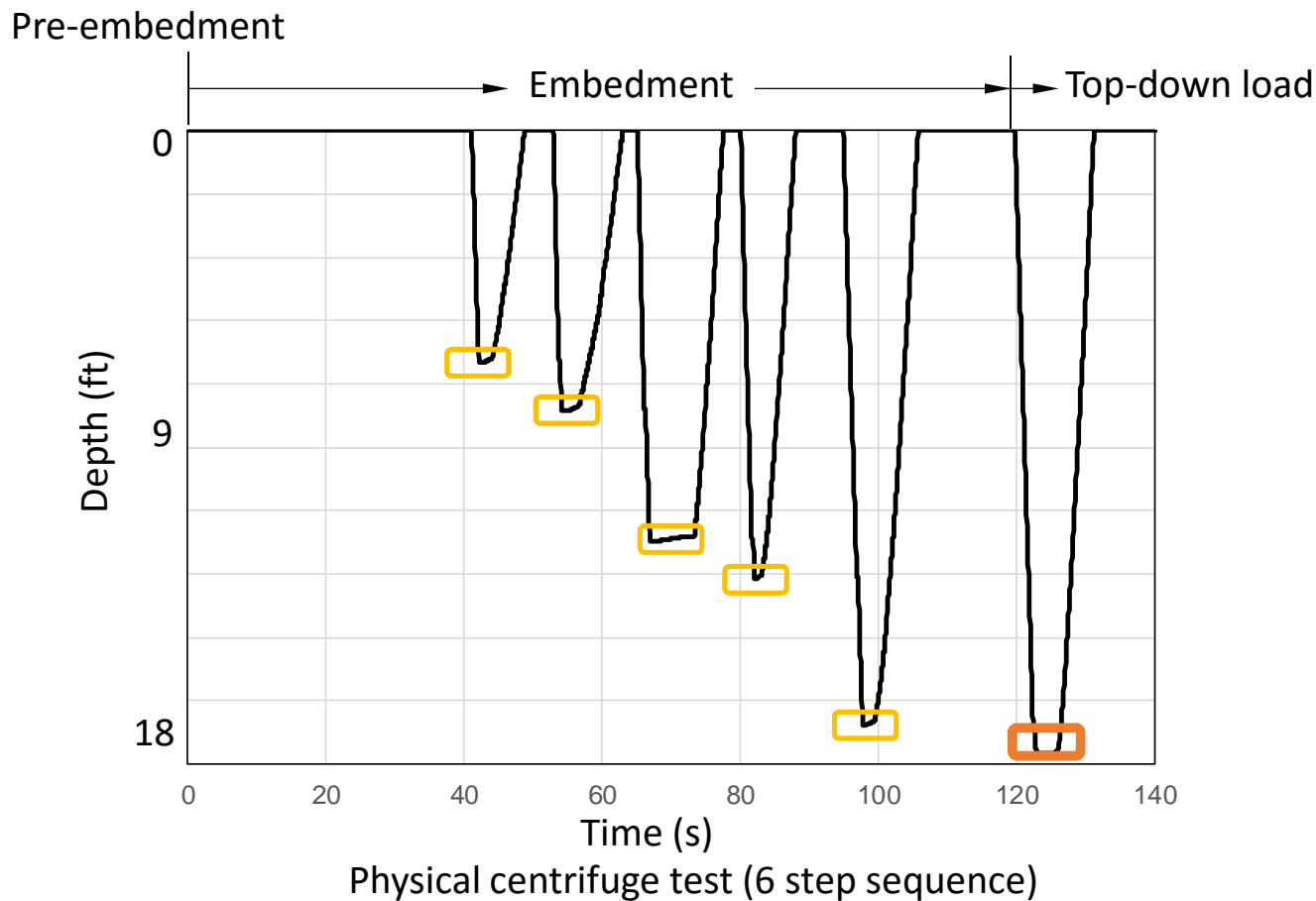
Z stress  
RED -> 0 psf  
BLUE -> 4200 psf



DEM-FEM model

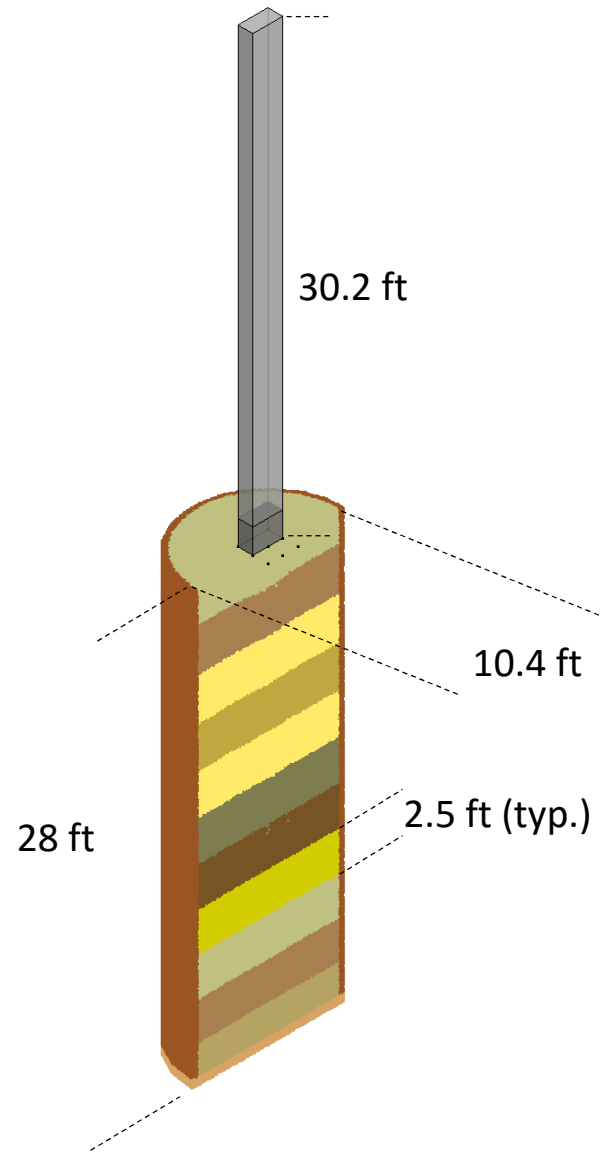
# DEM-FEM Modeling of Scenario 1

- Simulation stage 2:
  - Driving sequence based on physical test



# DEM-FEM Modeling of Scenario 1

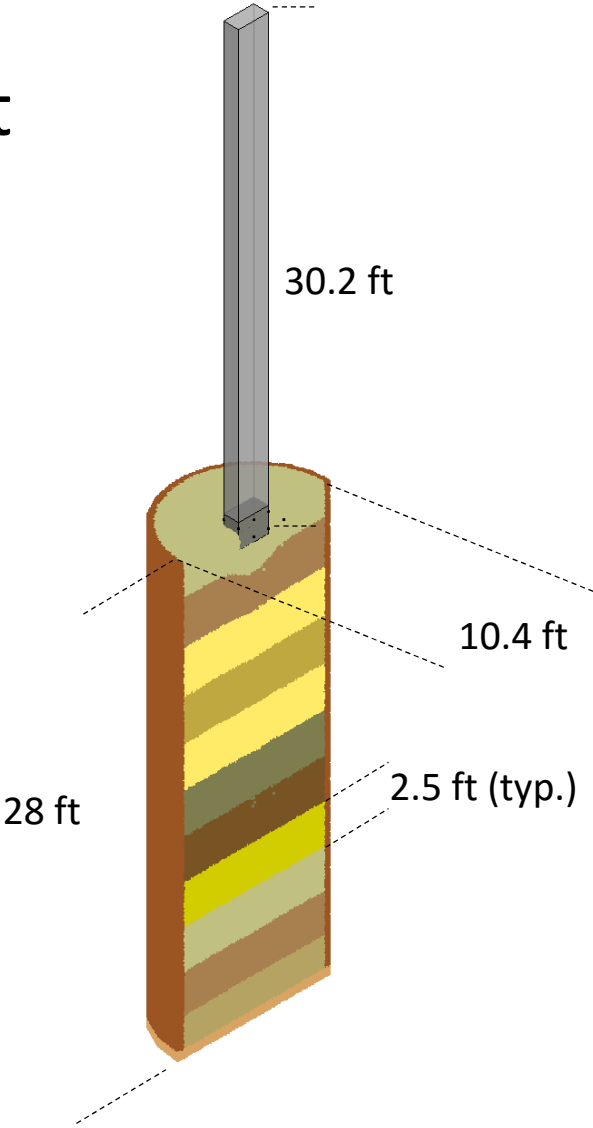
- Start of Stage 2
  - Pile tip depth:
    - 0 ft



Interior DEM assembly (elevation cutaway)

# DEM-FEM Modeling of Scenario 1

- Pre-embedment
  - Pile tip depth:
    - 1.3 ft

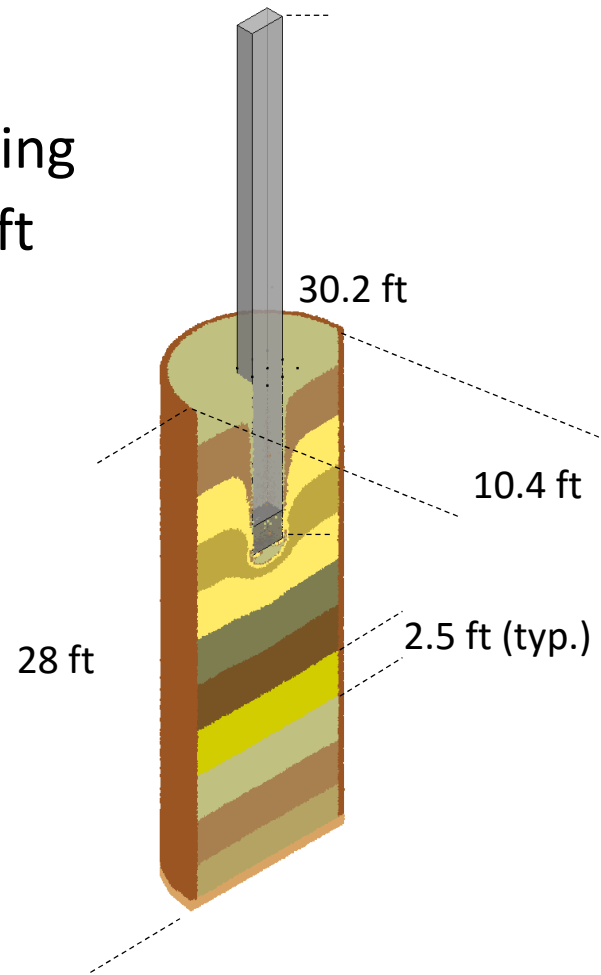


Interior DEM assembly (elevation cutaway)



# DEM-FEM Modeling of Scenario 1

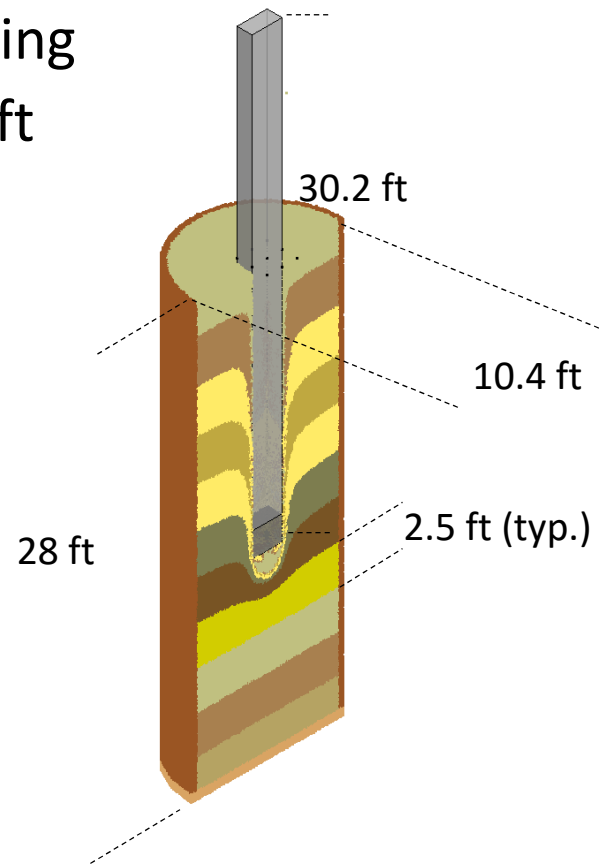
- Driving
  - Pile tip depth:
    - Matched staging
    - 1.3 ft to 17.8 ft



Interior DEM assembly (elevation cutaway)

# DEM-FEM Modeling of Scenario 1

- Driving
  - Pile tip depth:
    - Matched staging
    - 1.3 ft to 17.8 ft

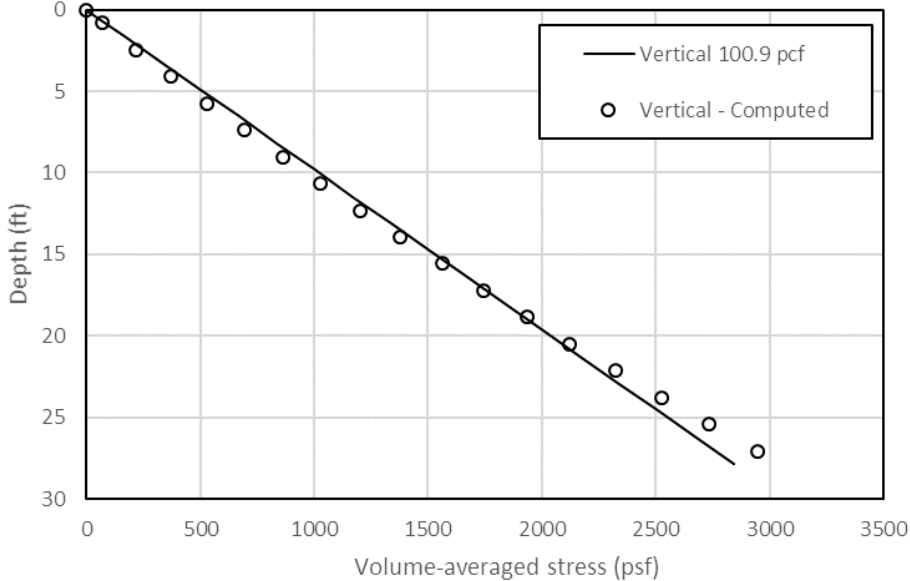
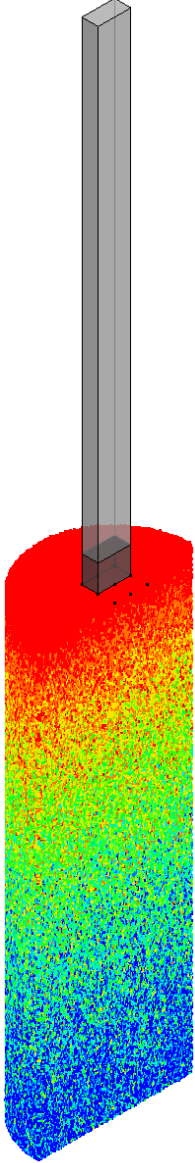
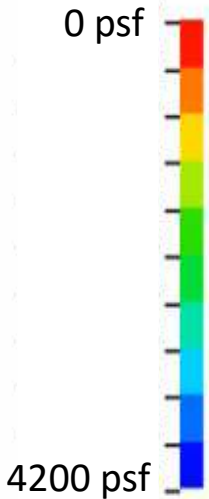


Interior DEM assembly (elevation cutaway)

# DEM-FEM Modeling of Scenario 1

- Computed stresses
  - Pile tip depth:
    - 0 ft

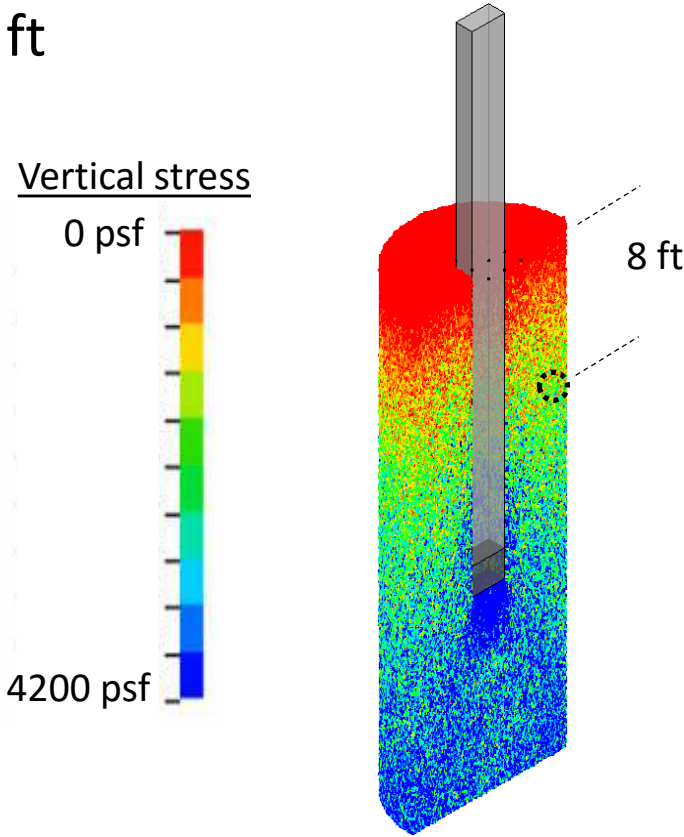
Vertical stress



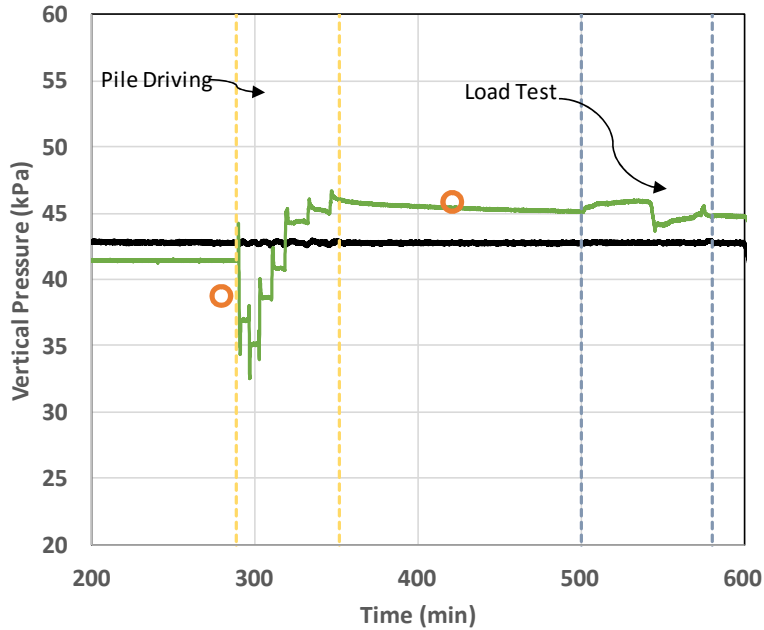
Interior DEM assembly (elevation cutaway)

# DEM-FEM Modeling of Scenario 1

- Computed stresses
  - Pile tip depth:
    - 17.8 ft



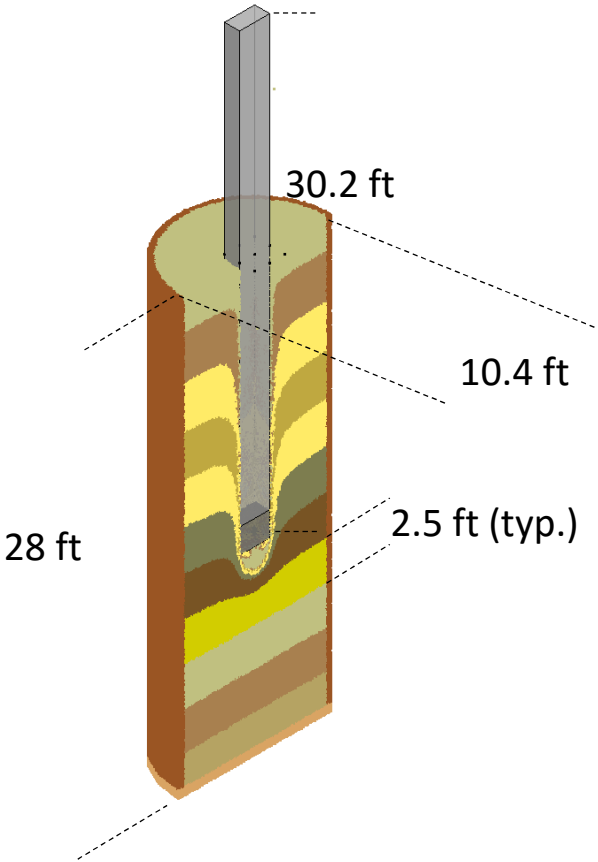
Computed vertical stress 800 psf to 960 psf  
Measured vertical stress 877 psf to 940 psf



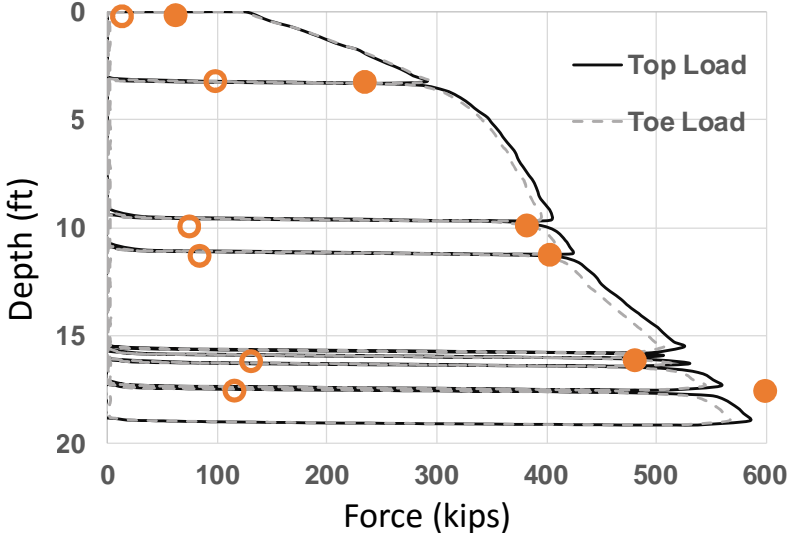
Interior DEM assembly (elevation cutaway)

# DEM-FEM Modeling of Scenario 1

- Preliminary load results during driving



Interior DEM assembly (elevation cutaway)



Measured vs computed envelopes

# Task 3. Numerical Modeling

- Next steps
  - Scenario 1: Perform top-down load test simulations and compare to physical test results (August - September 2017)
  - Scenarios 2-3: Incorporate SPW installation and removal into simulations (October - December 2017)



# Thank you.

