



Ground Tire Rubber (GTR) as a Stabilizer for Subgrade Soils

FDOT Contract Number: BDK81 977-03

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Problem Statement

-  GTR supplies may increase when not used in Hot Mix
-  Are other highway applications possible?



Outline



Objectives



Task overview



Results













Objective

 Determine the key pavement engineering properties of GTR and stabilized Florida subgrade soil blends



Tasks

-  Task 1 Literature Search
-  Task 2 Determine GTR Sources
-  Task 3 Determine Subgrade Sources
-  Task 4 Test Program Development
-  Task 5 Database Development
-  Task 6 Sampling
-  Task 7 Testing
-  Task 8 Data Reduction
-  Task 9 Data Analysis
-  Task 10 Technology Transfer



Literature Search

Density

-  Decreased with increase of GTR

LBR

-  Decreased with increase of GTR
-  Smaller sizes of GTR result in larger decreases of CBR/LBR

Resilient Modulus

-  Decreased with increase of GTR

Permeability

-  Increased slightly with maximum percentages of rubber

Consolidation

-  No literature on Consolidation of granular soils was found

Creep

-  Minimum failure strain at ~3%

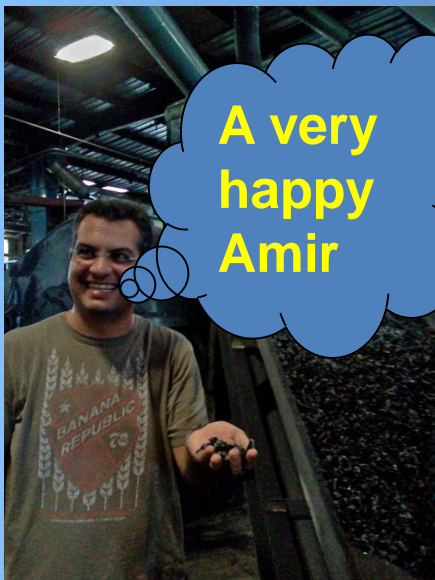


GTR Subgrade Choices

- 🐾 Three soil types (FDOT SMO Aided)
 - 🐾 Low LBR (20) – A-3
 - 🐾 Medium LBR (40) – A-2-4
 - 🐾 High LBR (80) – A-2-4
- 🐾 FDOT approved GTR supplier with three sizes
 - 🐾 1 inch (Range: 1-inch to 3/8-inch)
 - 🐾 3/8 inch (Range: 1/2-inch to #4 sieve)
 - 🐾 #40



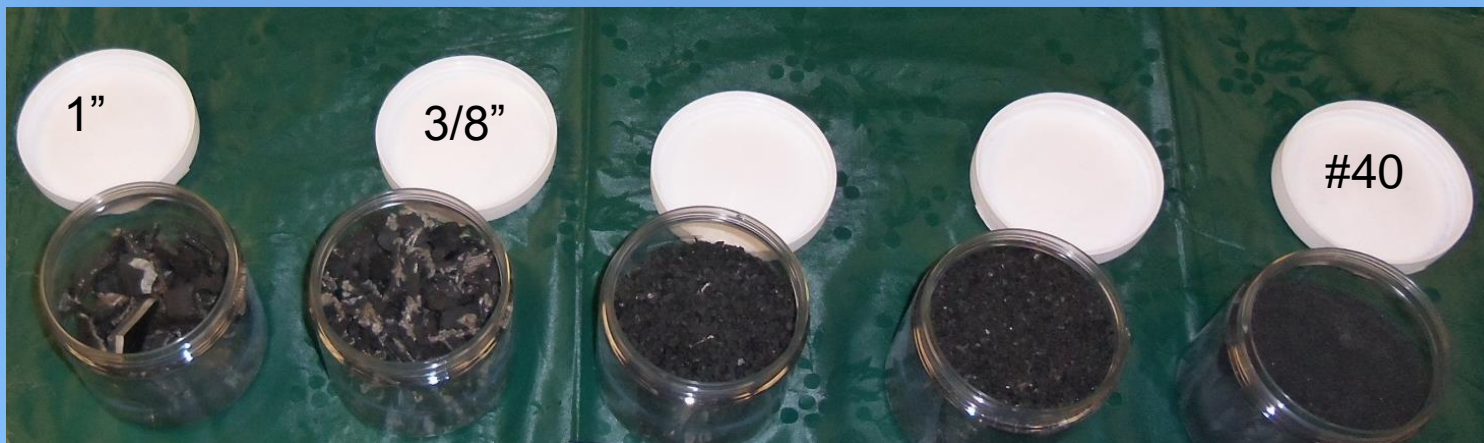
Global Tire Recycling Plant Site Visit



A very happy Amir



A very serious Alex





Testing Program

1. Atterberg Limits
2. Optimum Moisture Content
3. Sieve Analysis
4. Volumetric Mixing
5. LBR
6. Resilient Modulus
7. Creep
8. Permeability
9. Consolidation

Subgrade Only

The diagram consists of a vertical list of nine tests on the left. To the right of the list, there are two large blue curly braces. The top brace spans the first three items (Atterberg Limits, Optimum Moisture Content, Sieve Analysis) and is labeled 'Subgrade Only'. The bottom brace spans the remaining six items (Volumetric Mixing, LBR, Resilient Modulus, Creep, Permeability, Consolidation) and is labeled 'Subgrade GTR Blends'.

Subgrade GTR Blends



Atterberg limits

 Low LBR Subgrade

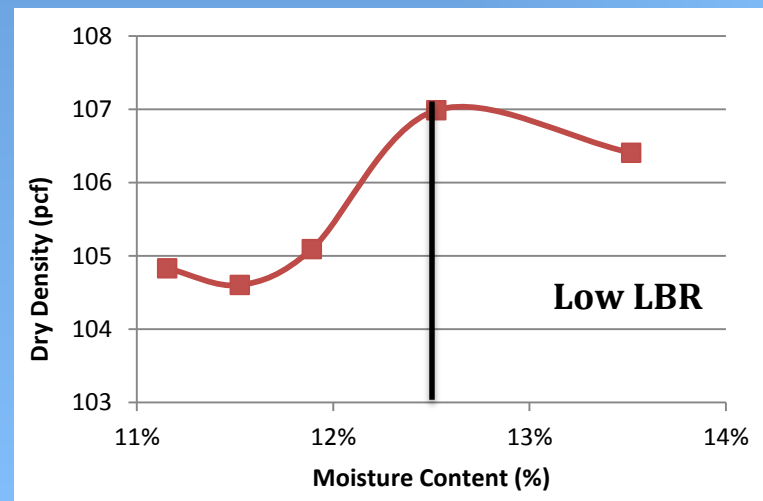
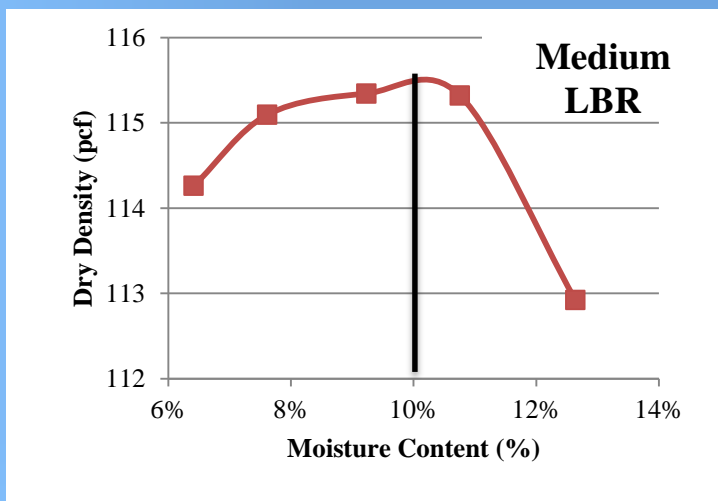
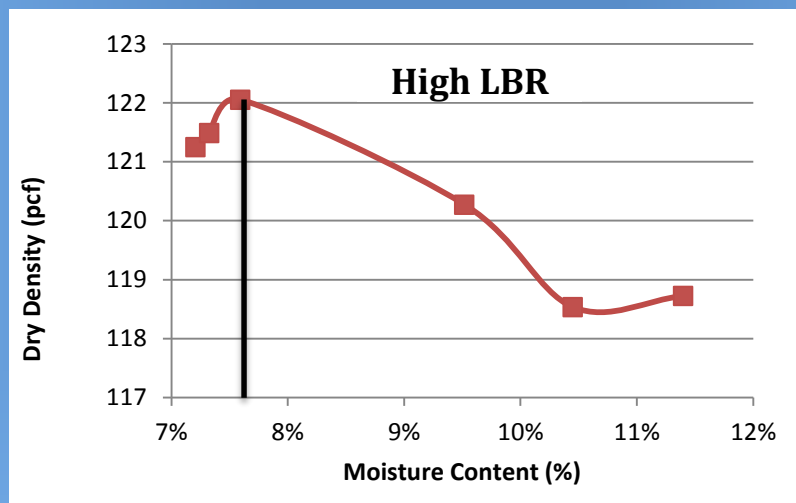
 No fines

 Medium & High LBR Subgrade

 No plastic fines



Moisture Density (Modified Proctor)



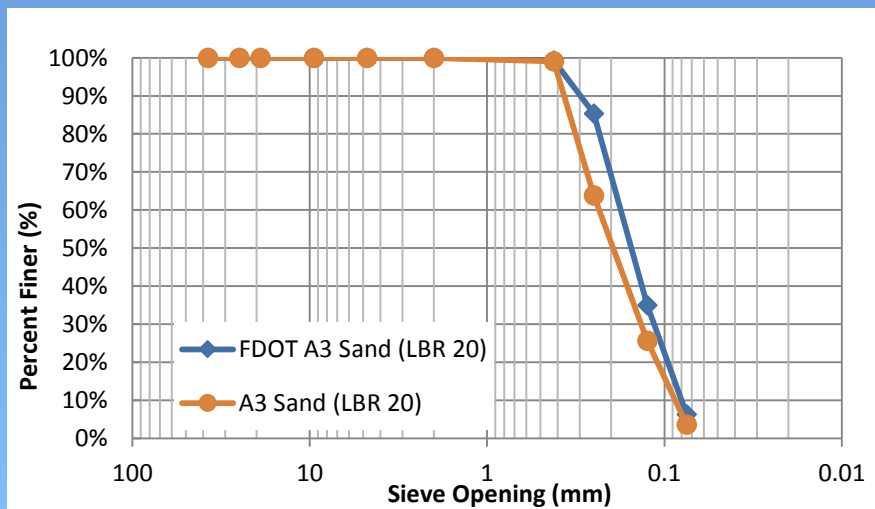
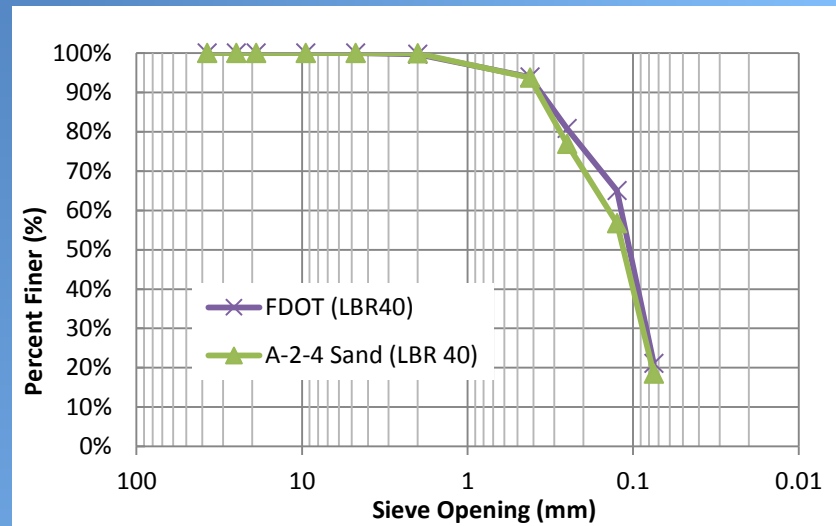
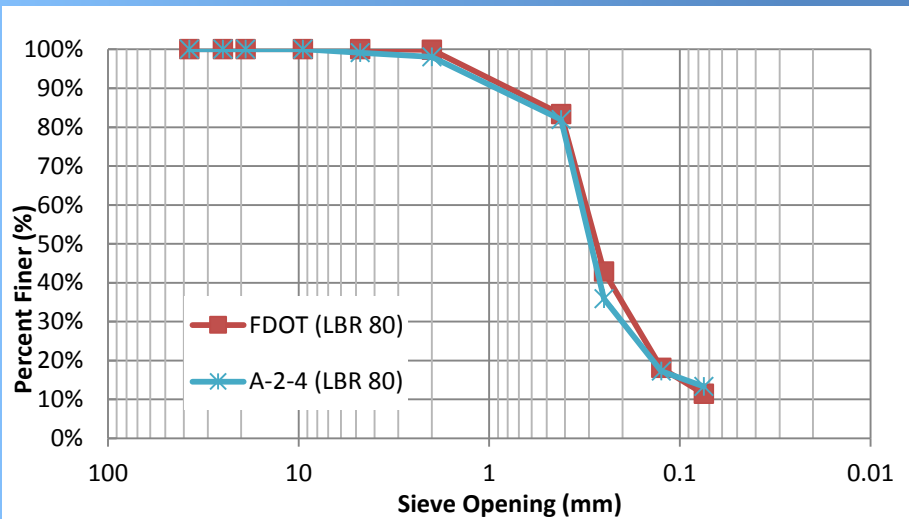


Test Results: Optimum Moisture Content

| Source | Maximum Dry Density | Optimum Moisture Content |
|------------|------------------------|--------------------------------|
| | (pcf) | (%) |
| Low LBR | 107 | 12.5% |
| Medium LBR | 115 | 10.0% |
| High LBR | 122 | 7.5% |



Sieve Analyses






Sieve Analysis Results

| Grain Size Characteristic | Low LBR Material | Medium LBR Material | High LBR Material |
|---------------------------|------------------|---------------------|-------------------|
| Uniformity Coefficient | 2.2 | 2.0 | 4.1 |
| Curvature Coefficient | 1.1 | 63.9 | 1.4 |
| Passing # 200 | 5% | 20% | 12% |
| AASHTO Classification | A-3 | A-2-4 | A-2-4 |
| USCS Classification | SP | SM | SM |



Volumetric Blending

 Mixing by volume used in the field

 4%, 8%, 16%, 24%, 32% GTR by volume

 Corresponds to

 1/2", 1", 2", 3" and 4" GTR layers in a 12" lift



Lab Blending Equivalences

| Soil Type | GTR % by Weight | GTR % by Volume |
|------------|-----------------|-----------------|
| High LBR | 1.1 | 4 |
| | 2.3 | 8 |
| | 4.7 | 16 |
| | 7.1 | 24 |
| | 9.7 | 32 |
| Medium LBR | 1.2 | 4 |
| | 2.4 | 8 |
| | 4.8 | 16 |
| | 7.4 | 24 |
| | 10.0 | 32 |
| Low LBR | 1.3 | 4 |
| | 2.6 | 8 |
| | 5.3 | 16 |
| | 8.0 | 24 |
| | 10.9 | 32 |

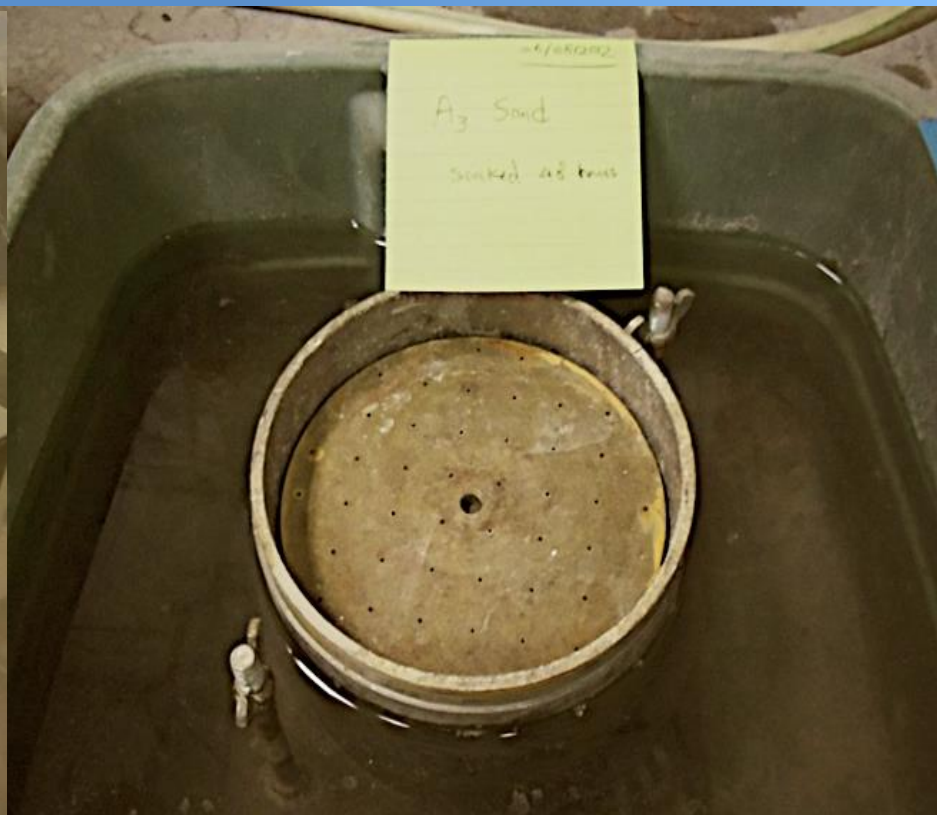


Blending





LBR





LBR (cont.)

 Limerock Bearing Ratio

 15 lb surcharge for subgrade





Limerock Bearing Ratio Results

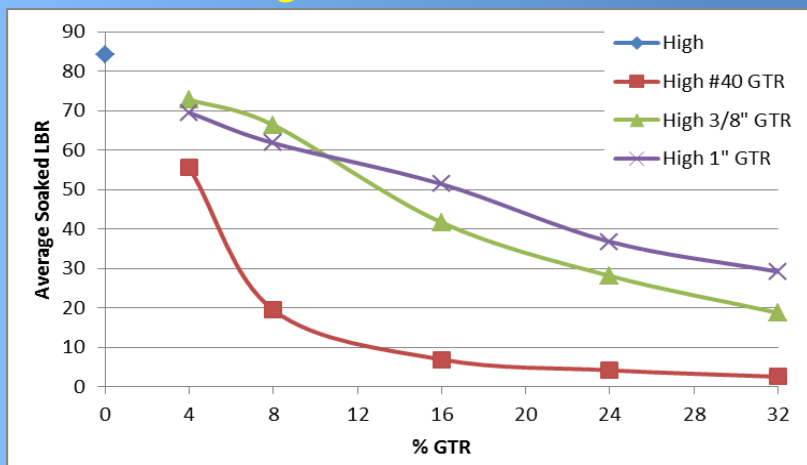
Subgrades

| Soil | Soaked LBR |
|----------|------------|
| High LBR | 88 |
| Med LBR | 38 |
| Low LBR | 20 |

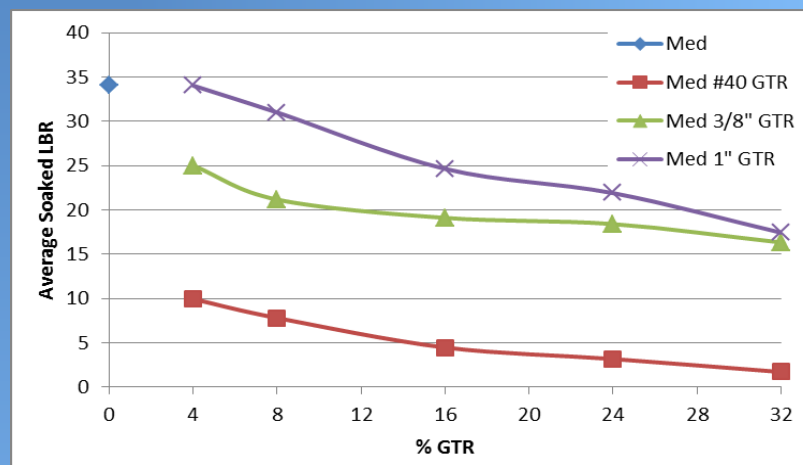


Limerock Bearing Ratio Results

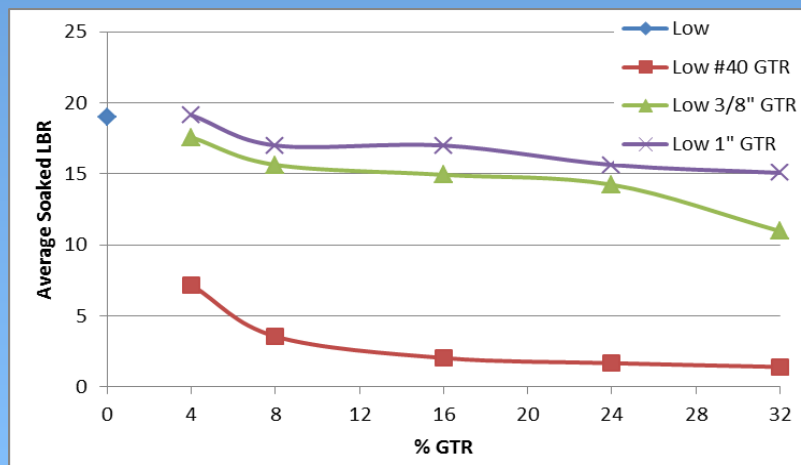
High LBR Blends



Medium LBR Blends



Low LBR Blends



All Decrease

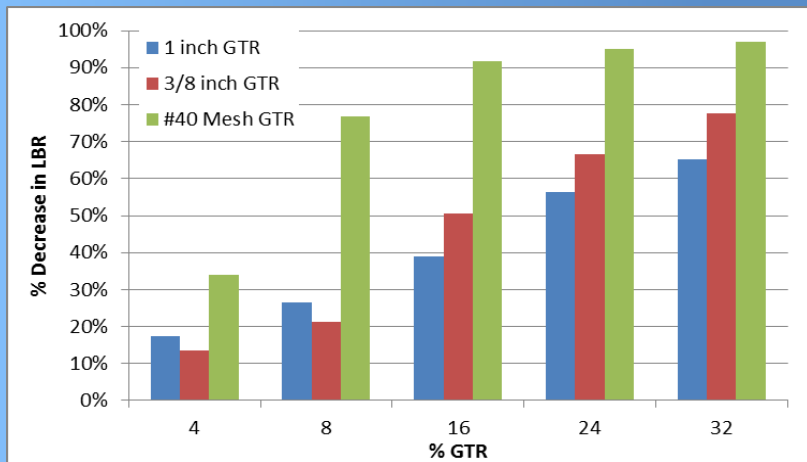


40 Blends Worst

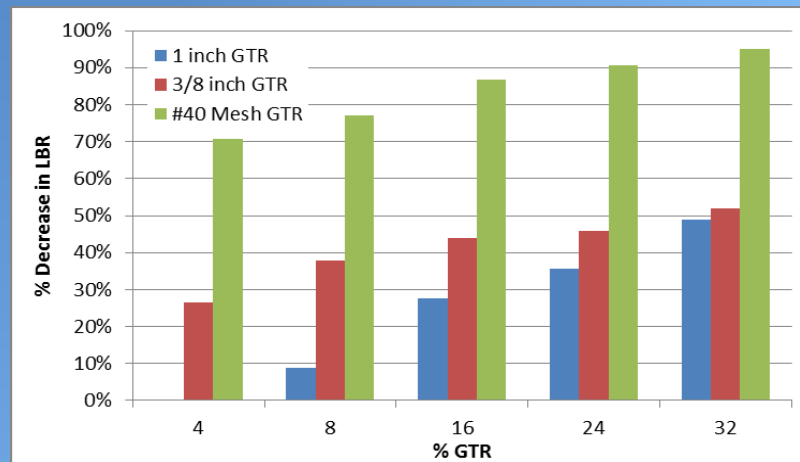


Limerock Bearing Ratio Results

High LBR Blends



Medium LBR Blends



Low LBR Blends



Largest Decrease # 40



Resilient Modulus

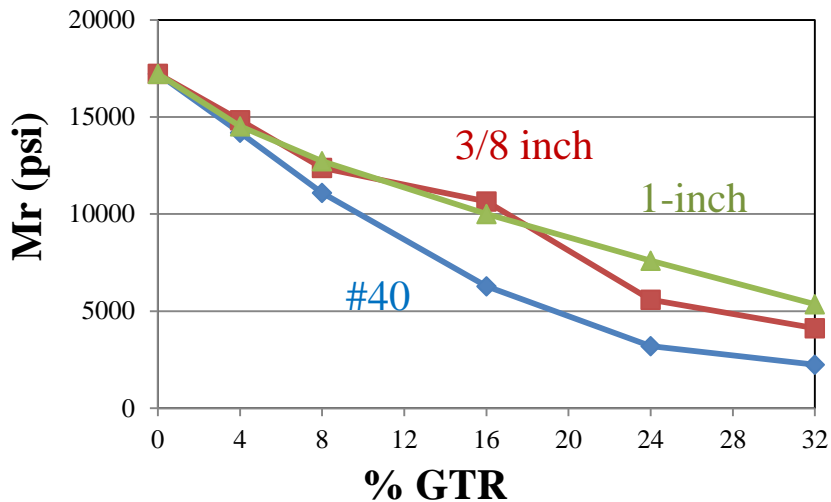
 Tests performed by the State Materials Office (SMO)



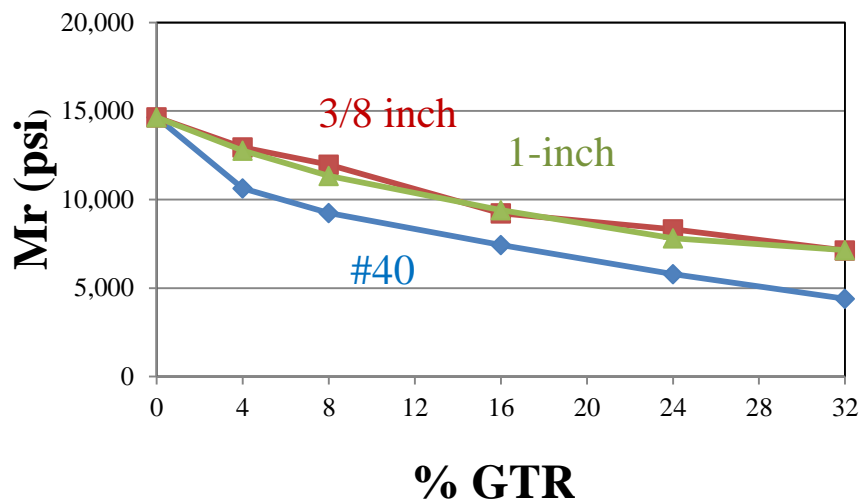


% GTR vs. Resilient Modulus

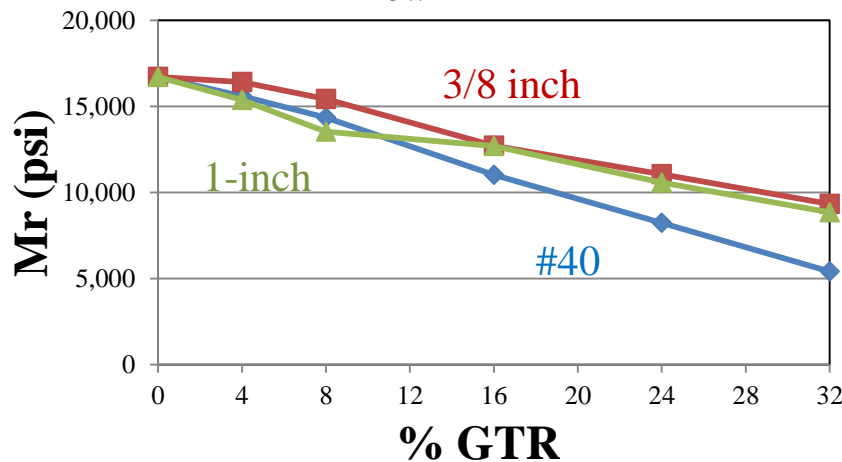
High LBR



Medium LBR



Low LBR



 All Decrease
 Low Mr Decrease < Medium LBR < High LBR



Creep

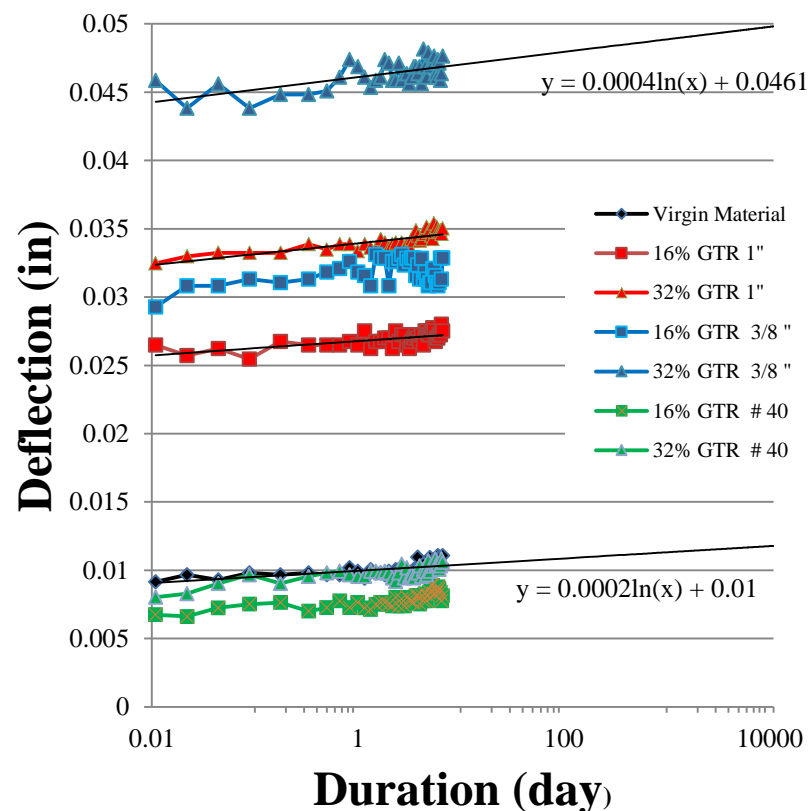
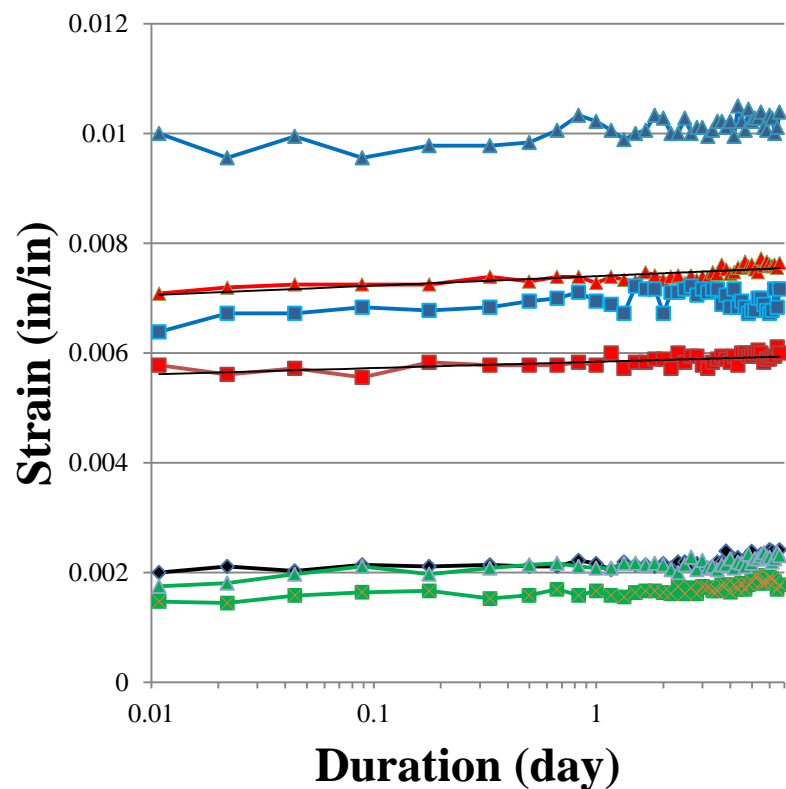




High LBR Material

30-Year Deflection Projection High LBR Material

Strain vs. Duration High LBR Material

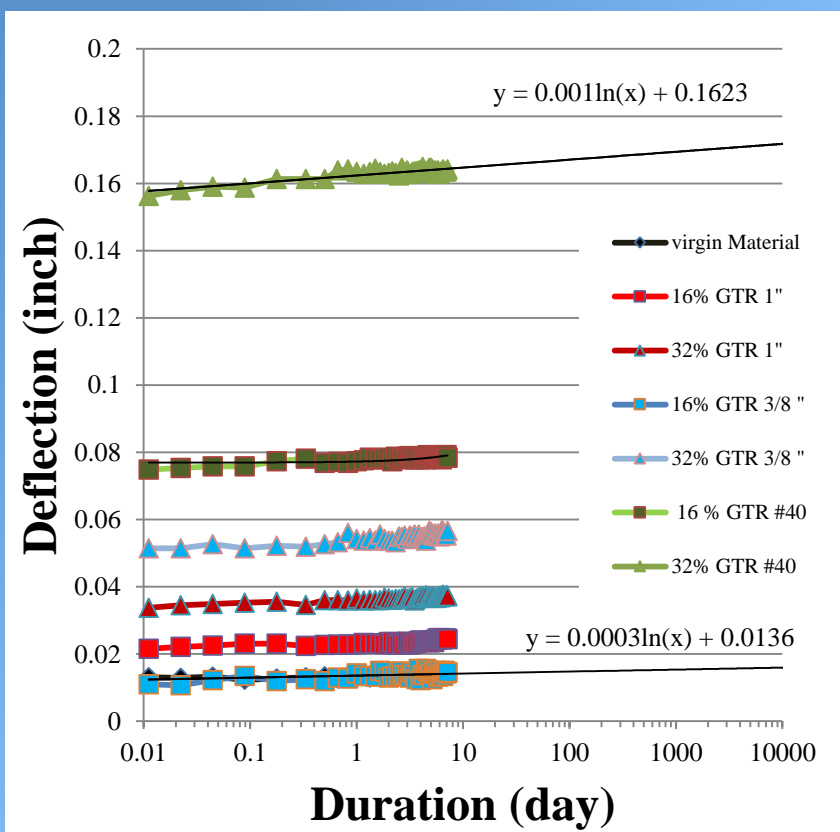
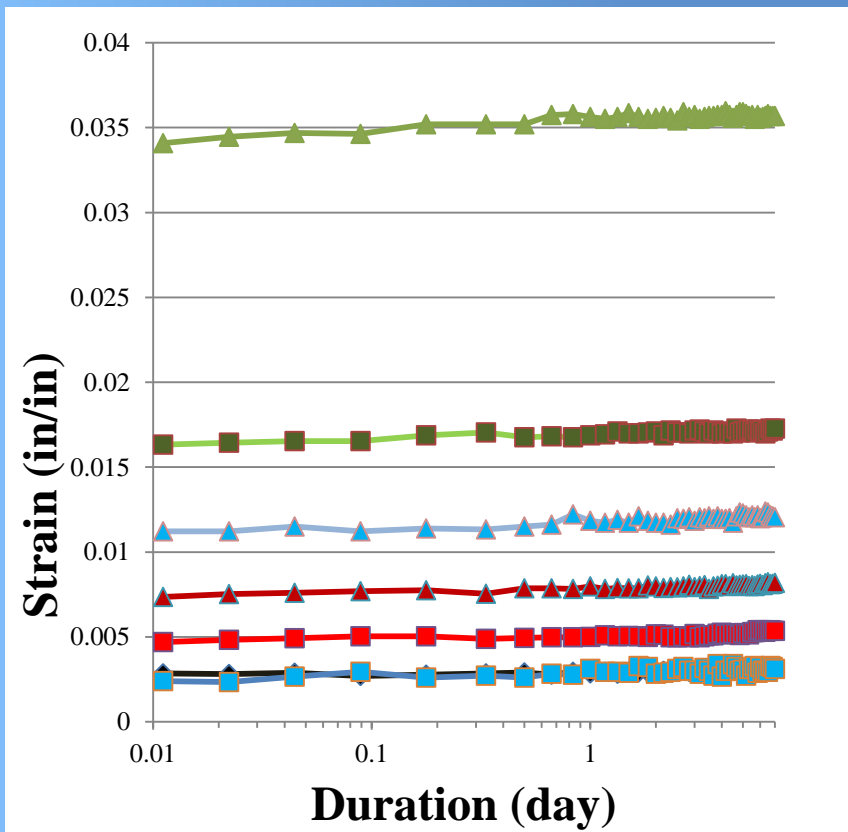




Medium LBR Material

30-Year Deflection Projection for Medium LBR Material

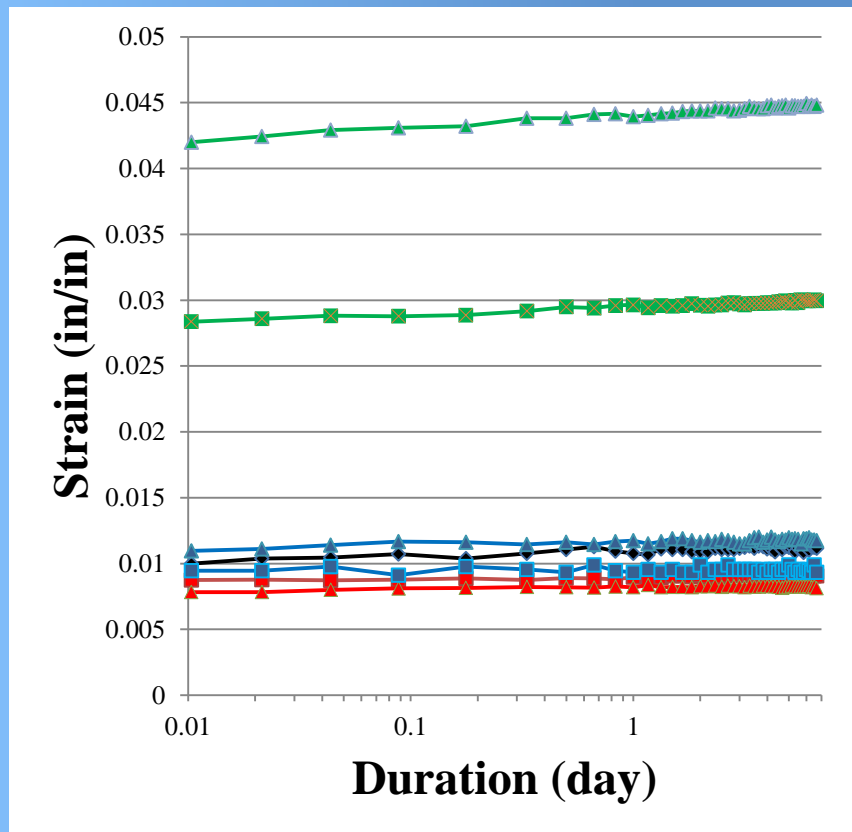
Strain vs. Duration Medium LBR Material



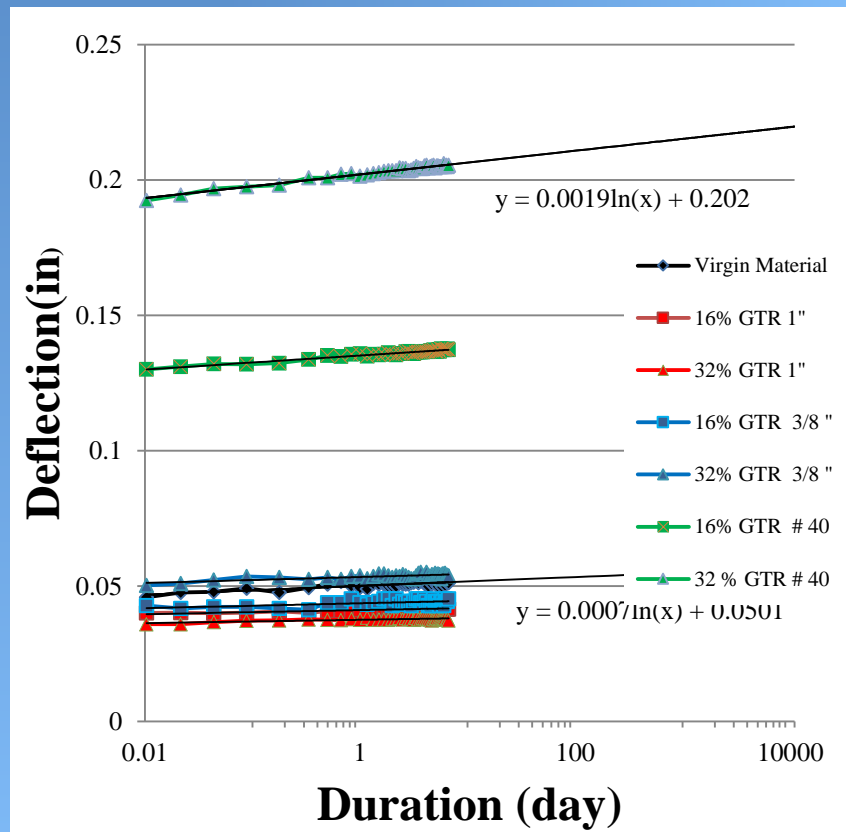


Low LBR Material

Strain vs. Duration Low LBR Material

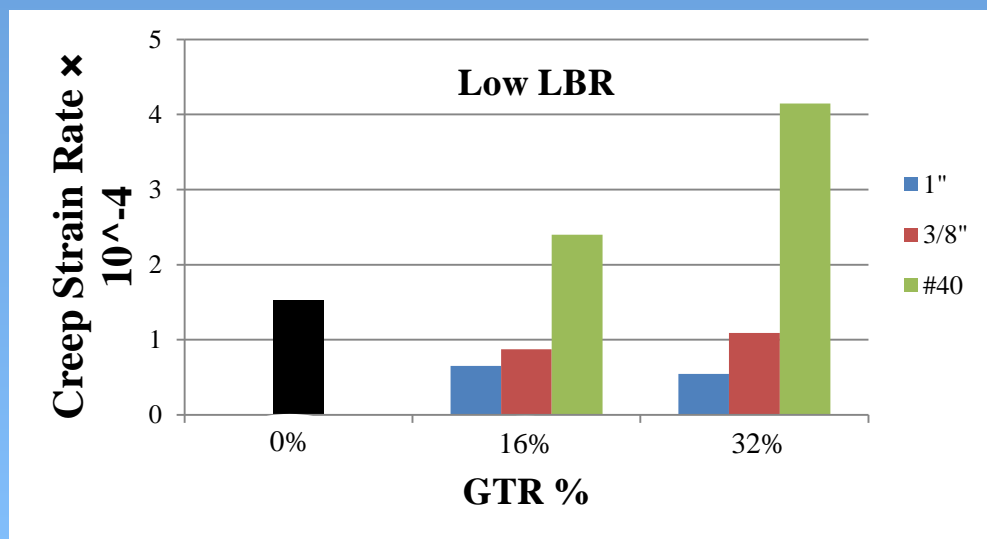
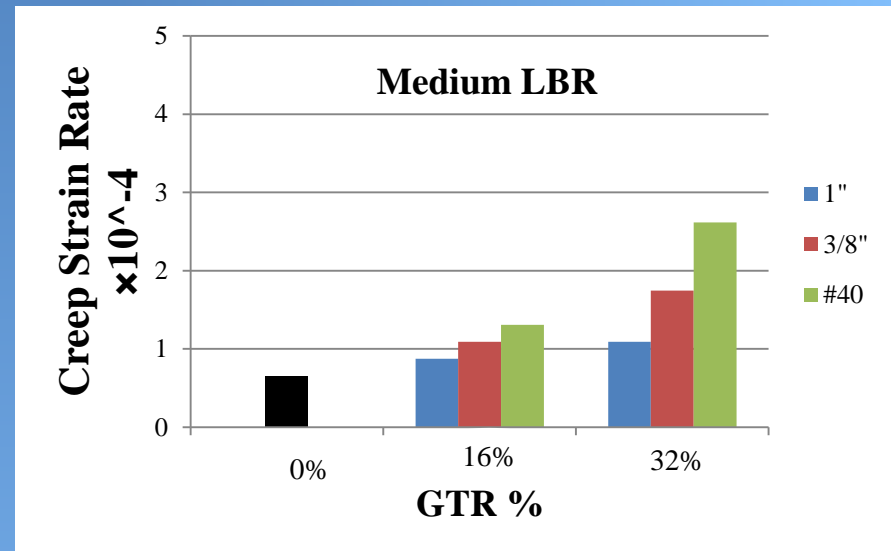
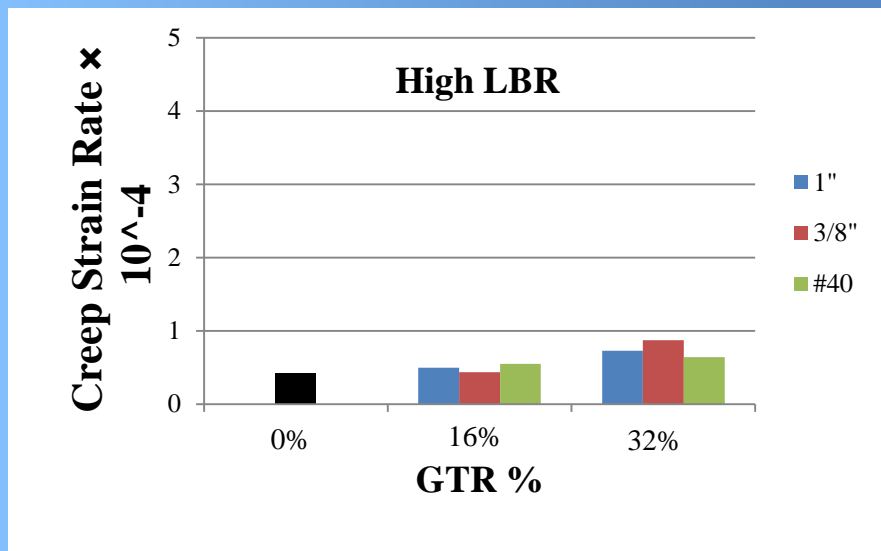


30-Year Deflection Projection for Low LBR Material





Strain Rate vs. GTR % for each Soil Type

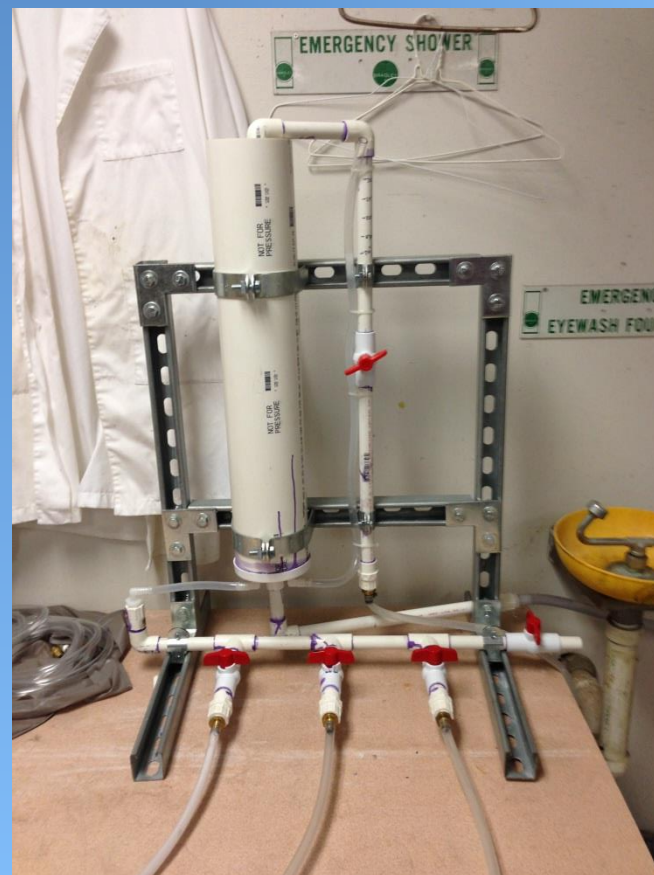


- Creep not a concern
- All relatively acceptable if 10^{-4}
- #40 with Low Material will produce 0.3 % strain over 30 years



Permeability

Constant Head Permeability Test Set-up





Test Results: Constant Head Permeability

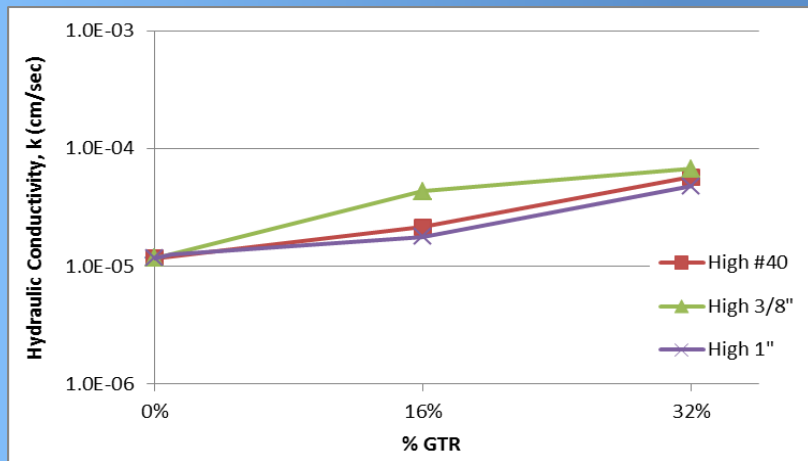
- Virgin Material
 - One Order of Magnitude Differences

| Soil | Hydraulic Conductivity, k (cm/sec) |
|------------|--|
| High LBR | 1.2×10^{-5} |
| Medium LBR | 2.8×10^{-6} |
| Low LBR | 3.7×10^{-4} |

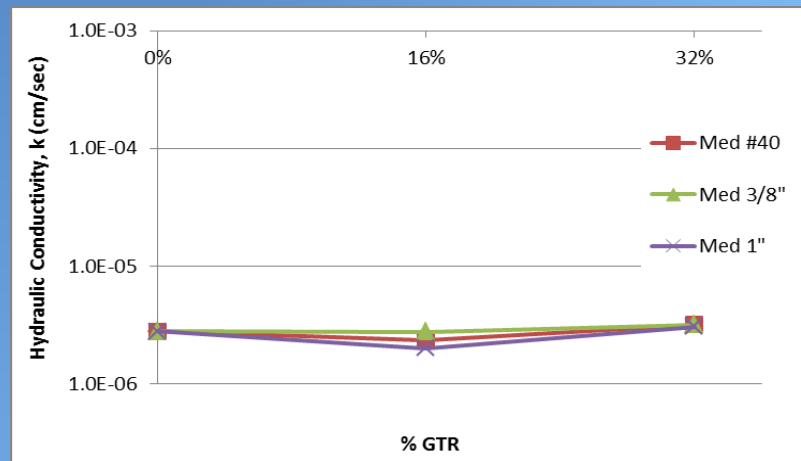


Constant Head Permeability

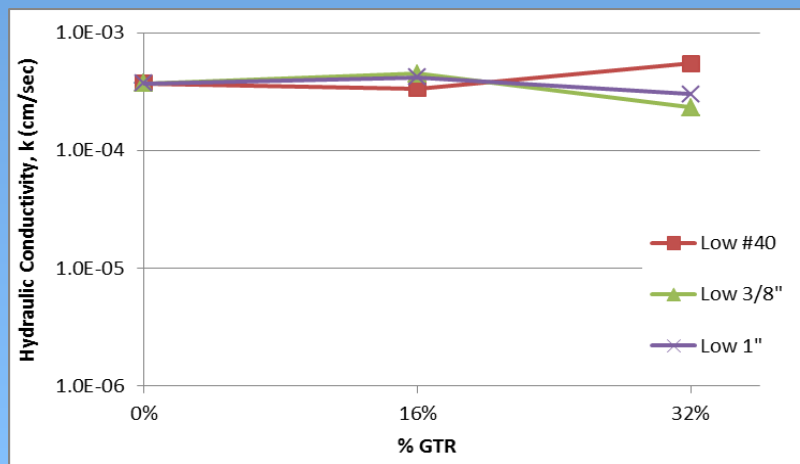
High LBR Blends



Medium LBR Blends



Low LBR Blends

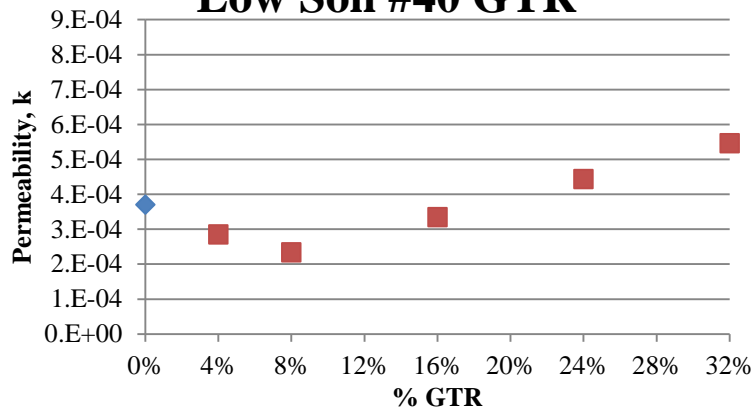


Very little change for all cases



Low LBR Material

Low Soil #40 GTR



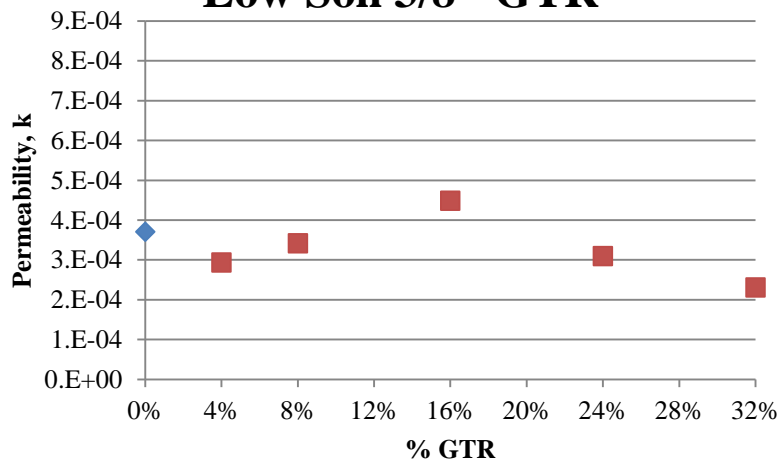
Typical results



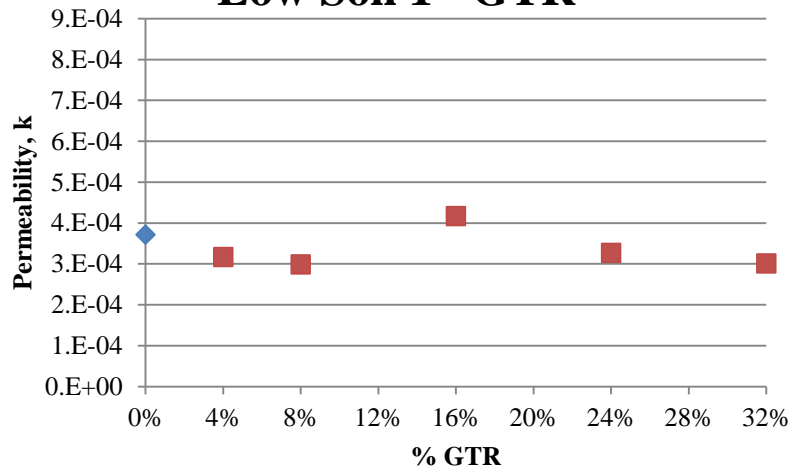
No significant change

| Soil | GTR | k (cm/sec) |
|----------|-----|------------|
| Low LBR | 0 | 3.7E-04 |
| Med LBR | 0 | 4.2E-06 |
| High LBR | 0 | 6.3E-06 |

Low Soil 3/8" GTR



Low Soil 1" GTR





Consolidation



Custom 4-inch Consolidation Molds





Consolidation

- Virgin Material Slope

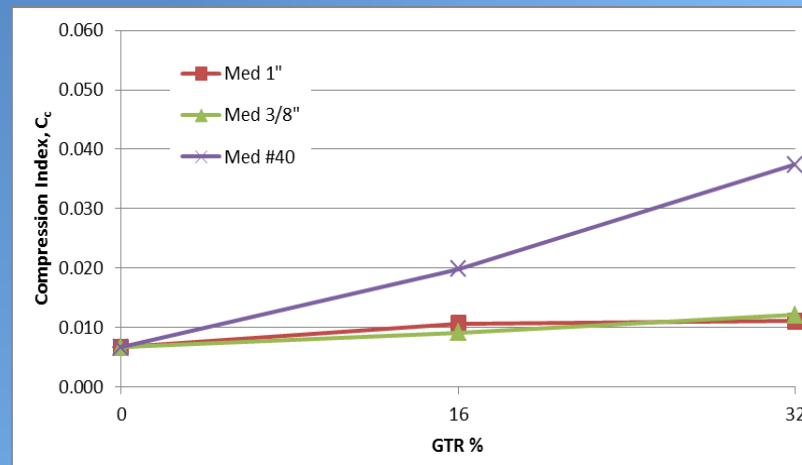
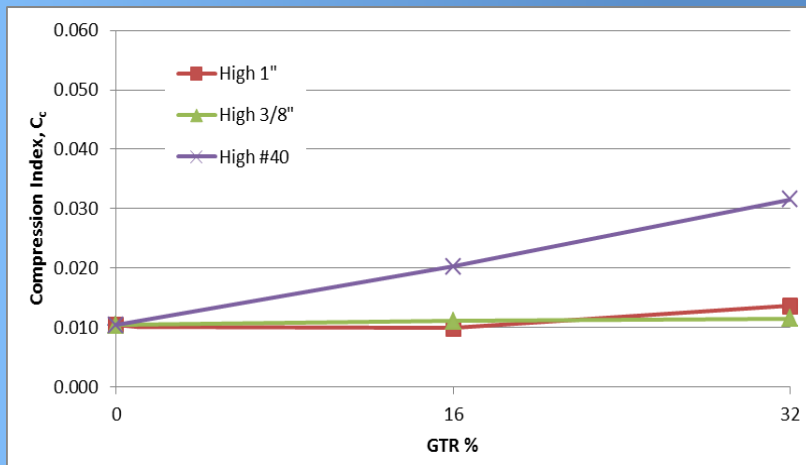
| Soil | Compression Index, C_c |
|----------|-----------------------------|
| High LBR | 0.010 |
| Med LBR | 0.007 |
| Low LBR | 0.008 |



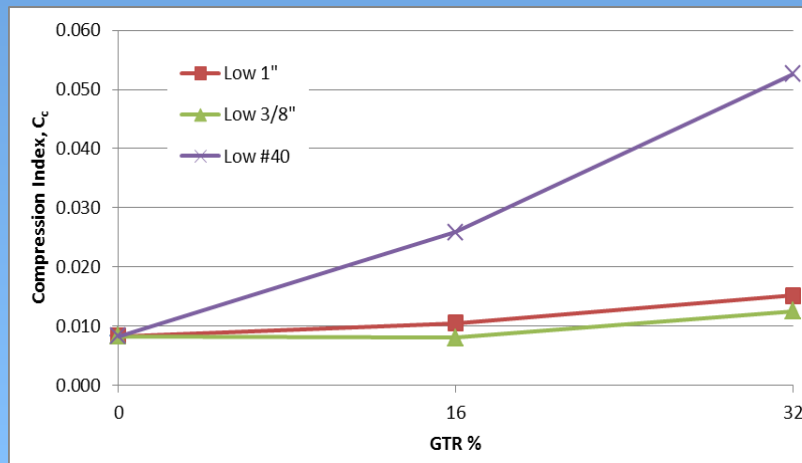
Consolidation Results

High LBR Blends

Medium LBR Blends



Low LBR Blends

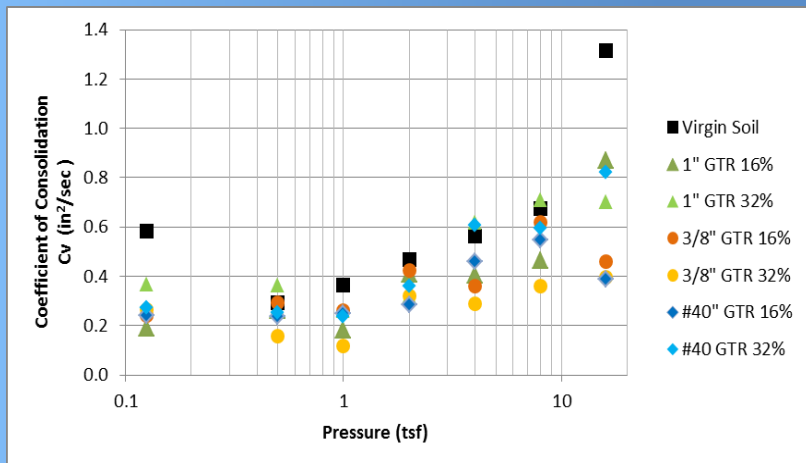


1" and 3/8" produce no change
 #40 blend causes change

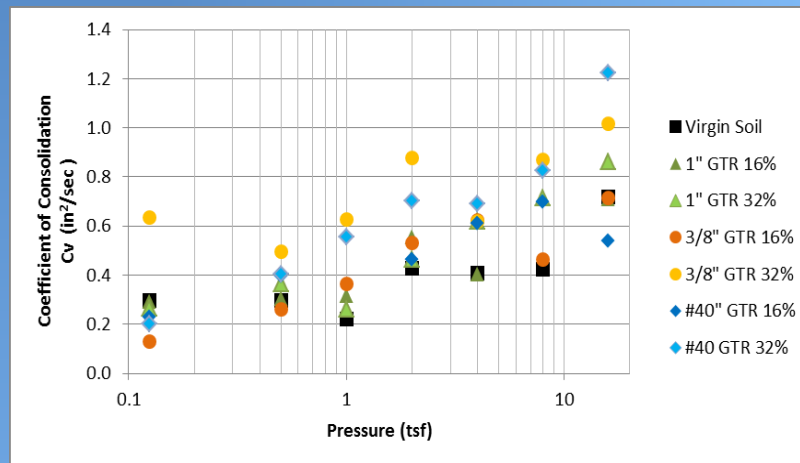


Consolidation Results

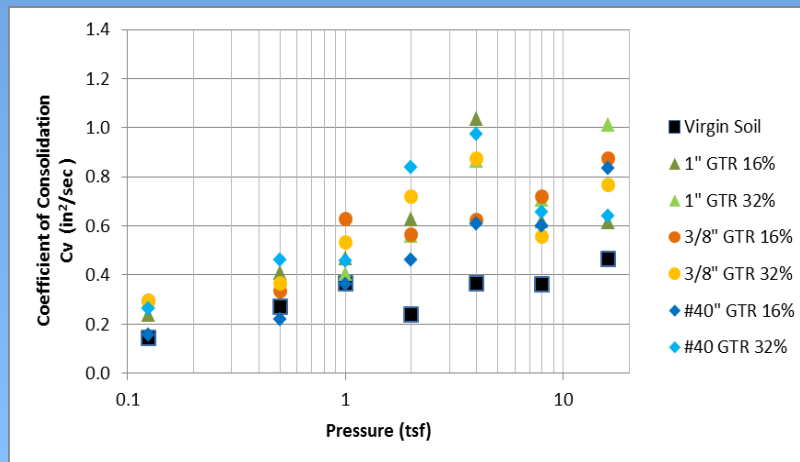
High LBR Blends



Medium LBR Blends



Low LBR Blends



No clear trends



Typical clays



10^{-3} to 10^{-4}



Much higher




Summary

- 🐾 With increasing GTR %:
 - 🐾 Density decreases
 - 🐾 LBR decreases
 - 🐾 Resilient Modulus decreases
 - 🐾 No significant Creep
 - 🐾 Not Consolidating
 - 🐾 No significant change in Permeability




Conclusions

 GTR Subgrade blends are not desirable for highway use

 LBR

 Decreases linearly with an increase of GTR

 #40 mesh GTR blends produced largest LBR decrease

 Low and Medium LBR subgrade blends were classified as unsuitable for use as a subgrade material

 High LBR subgrade blends with 1-inch GTR and 3/8-inch GTR produce acceptable LBR's up to 8% GTR by volume

 #40 GTR High LBR blends produce acceptable LBR's only at 4% GTR by volume






Conclusions

Constant Head Permeability



-  High LBR soil blends produce a small increase in k
-  Low and Medium LBR soil blends showed no significant k changes

Consolidation

-  Compressibility of 1" and 3/8" blends showed no change compared to virgin material
-  Compressibility of #40 mesh GTR blends increased by three to five magnitudes over the virgin material
-  C_v values in the soil/GTR blends were three to four orders of magnitude larger than typical remolded clays



Recommendations

-  Blends of High LBR Subgrade with minimal GTR concentrations could be suitable for the subgrade layer
-  Could be suitable as a possible lightweight, non-structural backfill due to decrease in density and increase in internal friction angle



Questions ?

