

Florida Department of Transportation

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August 3, 2011

This Memo Has Expired

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DCEMEMORANDUM 10-11

(FHWA Approved: 08/02/11)

TO: DISTRICT CONSTRUCTION ENGINEERS

FROM: David A. Sadler, Director, and Office of Construction

COPIES: Brian Blanchard, Robert Robertson, Chris Richter (FHWA), Bob Burleson (FTBA), Chad

Thompson, Abraham Scott, Rafiq Darji, Steve Plotkin, Andre Pavlov

SUBJECT: MSE WALL DESIGN STANDARDS INDEX 6000 SERIES

The Department recently met with members of the construction industry that manufacture and erect Mechanically Stabilized Earth (MSE) walls to listen to their concerns regarding the current design standards which result in the top panels of the walls having to almost all be cast to a custom height. This presents additional fabrication costs as well as additional costs associated with the tracking for correct installation on the project site.

As a result of the meeting with industry, the Department has agreed to make revisions that are intended to facilitate more tolerance in the coping recess which should aid in reducing the number of custom height panels at the top of the MSE walls. The revisions also include clarification of several notes.

The revised drawings are provided in Attachment A with the changes denoted by clouding. These revisions will also be included in the January 2012 release of the Design Standards e-Booklet publication effective for lettings beginning July 2012.

This memorandum serves as blanket approval to process a \$0.00 contract change, and a copy of this memorandum should be attached to the work order or supplemental agreement. If requested by the contractor to use this change, these revised details shall be included in the shop drawings.

If you have any questions concerning this memorandum or would like to request the MicroStation drawings shown in Attachment A, please contact Steve Plotkin at (904) 360-5501.

DS/ph Attachment

NOTES

SPECIFICATIONS:

- 1. General Specifications:
- The Florida Department of Transportation "Standard Specifications for Road and Bridge Construction", Current Edition and Supplements as Amended.
 2. Design Specifications:
- Floride Department of Transportation (FDOT) "Structures Design Guidelines",
- b. American Association of State Highway and Transportation Officials (AASHTO)
 "LRFD Bridge Design Specifications", Current Edition.
 c. AASHTO-AGC-ARTBA Task Force 27 (Ground Modification Techniques),
- "Insitu Soil Improvement Techniques", January 1990.

DESIGN CRITERIA:

- Design is based on the assumption that the meterial contained within the reinforced soil volume, methods of construction and quality of prefabricated materies are in accordance with Spacification Section 548 and Chapter 3 of the FDOY's Structures Design Guidelines.
- It is the responsibility of the Engineer of Record to determine that the maximum factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer of Record.
- If there are manholes and/ or drop inlets present, design and analysis for both internal and external stability shall be considered.

50IL PARAMETERS:

- See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

MATERIALS:

- 1. Concrete Class: See Wall Control Drawings.
- 2. See Specification Section 548 for material requirements.
- 3. For additional material requirements see the Wall Company's General Hotes.

CONSTRUCTION:

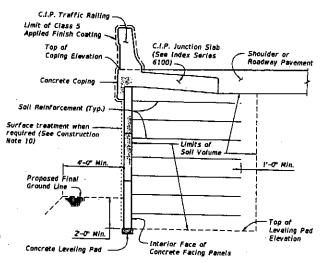
- Walls will be constructed in accordance with Specification Section 548 and the Wall Company's Instructions.
- For location and alignment of retaining walls, see Wall Control Drawings.
 If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.
- Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistances, minimum wall embedment and anticipated long term and differential sattlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (13° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrali posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 9. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are amitripated.

- All exposed concrete surfaces will receive a Class 5 Applied Finish Coating
 in accordance with Specification Section 400, Refer to Typical Section on
 this sheet and the following notes for limits of applied finish:

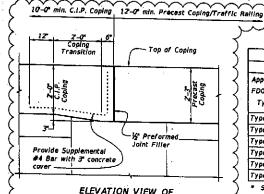
 The Inside, backside and top of Traffic Railings and Pedestrian/Bicycle
 Railings.
 - Exposed surfaces of coping on top of retaining well. Other coetings, colors or textures will be applied as required in the Well Control Drawings.
- For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6' below final ground line.
- 11. Drive piles located within the soil volume prior to construction of the retaining well, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shefts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 450
- 12. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- Steps in leveling pads will occur at MSE Well panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- The top of the leveling pad or footing will be 2-0" minimum below final ground line.
- 15. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 16. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 17. Work this Index with Index 6100 & 6200 Series.

SHOP DRAWING REQUIREMENTS:

See Specification Section 548 for shop drawing requirements.



TYPICAL MSE RETAINING WALL SECTION WITH A TRAFFIC RAILING (Showing Limits of the Reinforced Soil Volume)

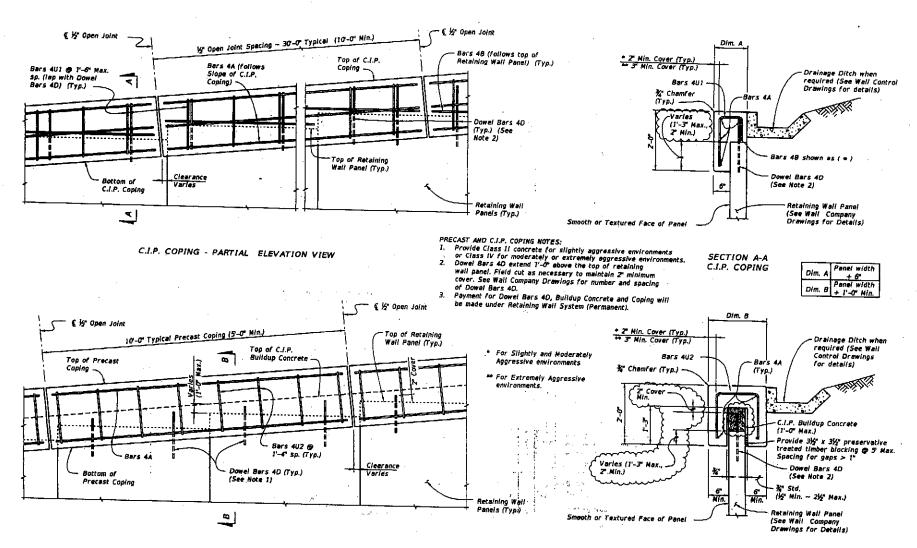


ELEVATION VIEW OF COPING HEIGHT TRANSITION (Railing Not Shown For Clarity)

	FDOT	MSE RET	AINING V	NALL CLASSII	ICA	rion	TAI	9) F		
	Dura	bility Requi	rements						Wall	Types
Applicable FDOT Wall Type *	Concrete Cover (In.)	Concrete Class for Panels	Pozzolen Additions?	Soil Reinforcement Type	ZA	28	20	20	2E	2F
Type 2A	2 .	и	No	Netal	┪	-		_	-	_
Type 2B	2	IV .	Na	Metal	 	<u> </u>	-	1	-	
Type 2C	3	IV	No	Netai			_	<u> </u>	1	
Type 2D	3	IV	Yes	Metal			_	 - -	-	
Type 2E	3	/V	No	Plastic	 			 	l ´	<u> </u>
Type 2F	3	IV	Yes	Plastic	t	 		 	 	

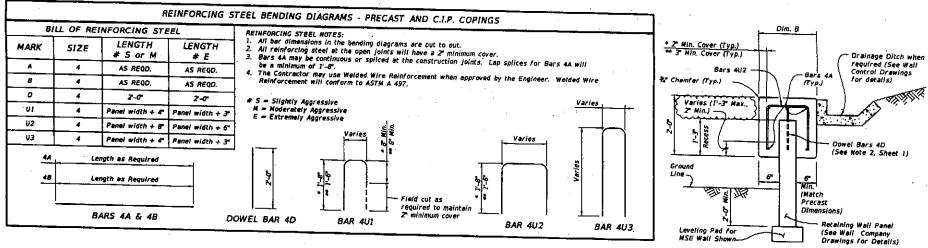
- * See Data Table in Contract Plans.
- ** Silica fume, metakaolin or ultrafine fly ash.

GENERAL NOTES AND DETAILS



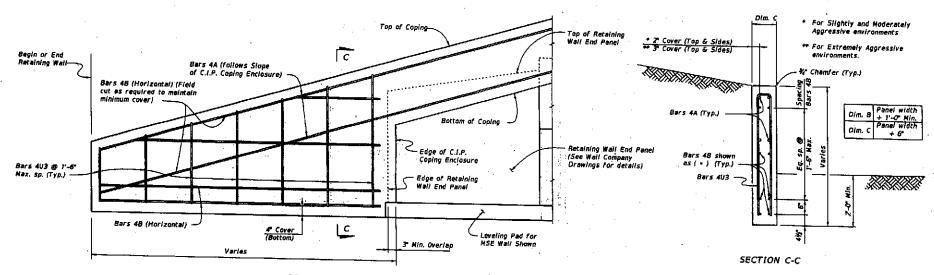
PRECAST COPING - PARTIAL ELEVATION VIEW

SECTION B-B PRECAST COPING

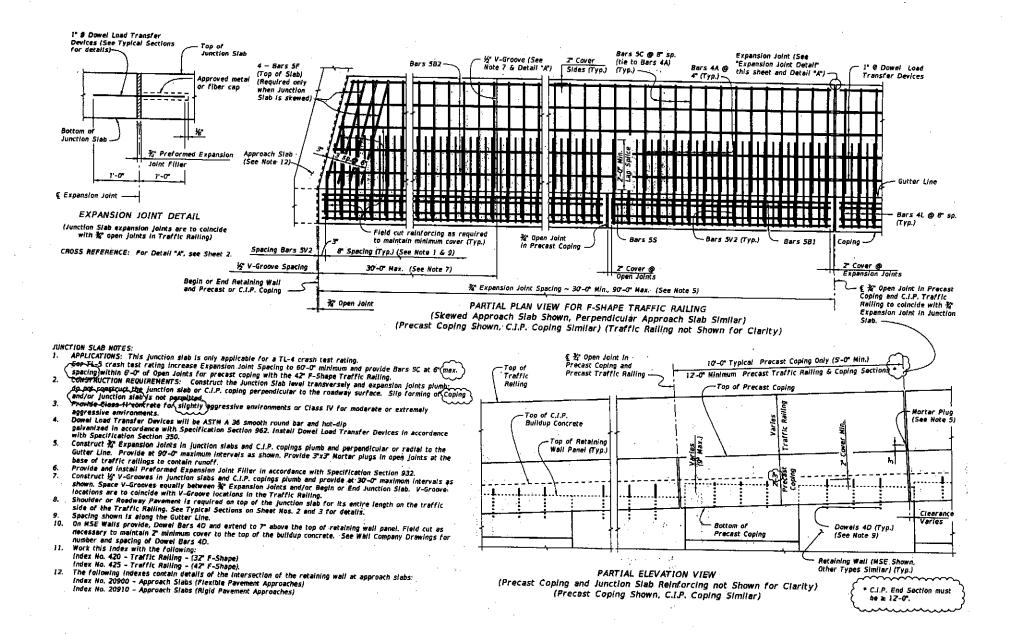


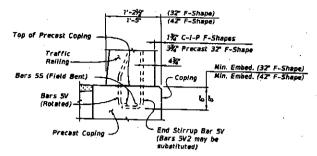
C.I.P. COPING USED WITH PRECAST COPING

Note: When precast coping units do not fit the entire length of the retaining wall, use this similar C.I.P. coping for short portions between precast coping units. This C.I.P. coping may also be used for vertical copings.



C.I.P. COPING ENCLOSURE DETAIL



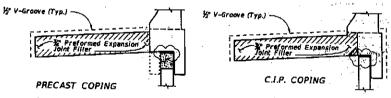


PARTIAL END VIEW OF TRAFFIC RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 5V and Bars 5S) (Precast Coping Shown, C.I.P. Coping Similar)

NOTE: See Index No. 420 and Index No. 425, Detail "A" for details.

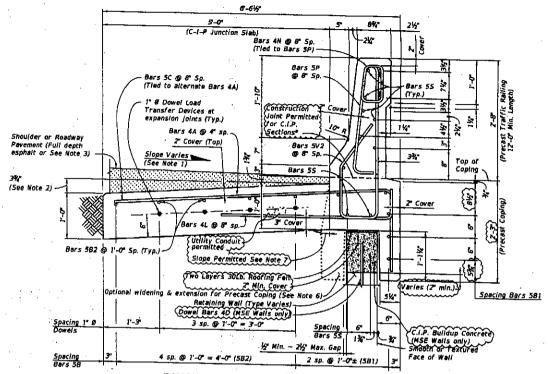
ESTIMATED QUANTITIES FOR	PRECAST	COPING
ITEM	UNIT	QUANTITY
Concrete (Pracast Coping Only)	CY/FT	0.079
Concrete (Precest Barrier & Coping)	CY/FT	0.165
Concrete (C.I.P. Junction Siab)	CY/FT	0.185
Reinforcing Steel (Precast Coping & Traffic Railing)	L8/FT	(52.67)
Reinforcing Steel (C.I.P. Junction Slab) (Typ.)	LB/FT	12.52
Additional Reinf. @ Expansion Joints	LB	21.36

(The above concrete quantities are based on a max, superelevation of 6.25% and a 32" F-Shape Traffic Railing.



DETAIL "A"

(Showing Locations of 1/2 V-Grooves and 1/2 Preformed Expansion Joint Filler)



TYPICAL SECTION THRU PRECAST* 32" F-SHAPE TRAFFIC RAILING AND COPING WITH C-I-P JUNCTION SLAB

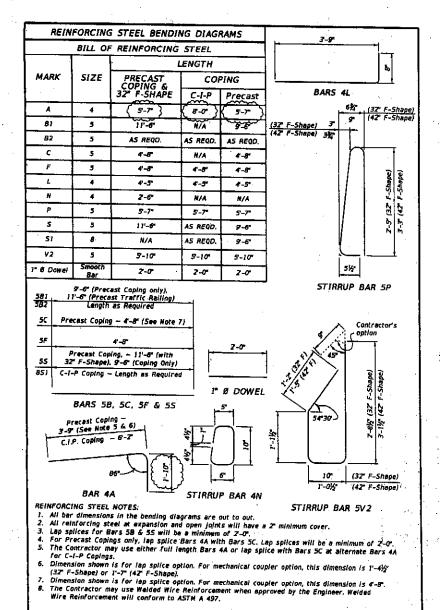
C.F. Traffic Halling and Coping Sections are permitted at End Sections. Prainage Inlets and Light Pole Padestals if slip forming is not used.

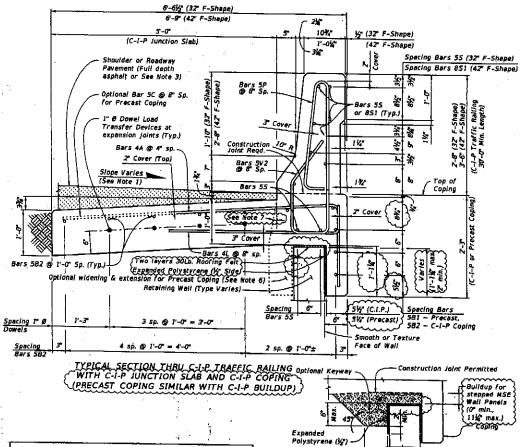
- 1. Match Cross Slope of Travel Lane or Shoulder.
- 2. The 3% dimension corresponds to a maximum superelevation of 6.25%. For steeper superelevations increase this dimension to match roadway
- 3. For Rigid Pavement (Concrete), Junction Siab may be thickened to match finish grade.
- 4. Minimum length of Junction Siab between expansion joints is 30°-0°.

 At the Contractor's option, mechanical couplers may be used to splice
- reinforcing. Complete details, including reinforcing the used to splice in the state of the stat
- yield strength.

 6. Contractor to maintain stability of precent cooling/traffic railing orior to junction stab completion. In the Shop Drawings, show reinforcement for coptional extension required for stability shipping and handling. Maintain 7. Angle varies — 0 min., 20 m

F-SHAPE TRAFFIC RAILINGS





ESTIMATED QUANTITIES FOR C.I.P. COPING

ITEM	UNIT	OUANTITY
Concrete (Traffic Railing not Included)	CY/FT	0.264
Reinforcing Steel (Typical) excluding Bars 5V2 and 55 (Typ.)	LB/FT	(30,89)
Additional Reinf. @ Expansion Joint	LB/FT	21.36

(The above concrete quantities are based on a max, superelevation of 6.25%, beneath a 32" F-Shape Traffic Railing on an MSE Wall).

NOTES:

NOTES:

AND C-I-P COPING

MACH Cross Slope of Travel Lane of Shoulder.

The 3% dimension corresponds to a maximum superelevation of 6.25%. For steeper superelevations increase this dimension to match

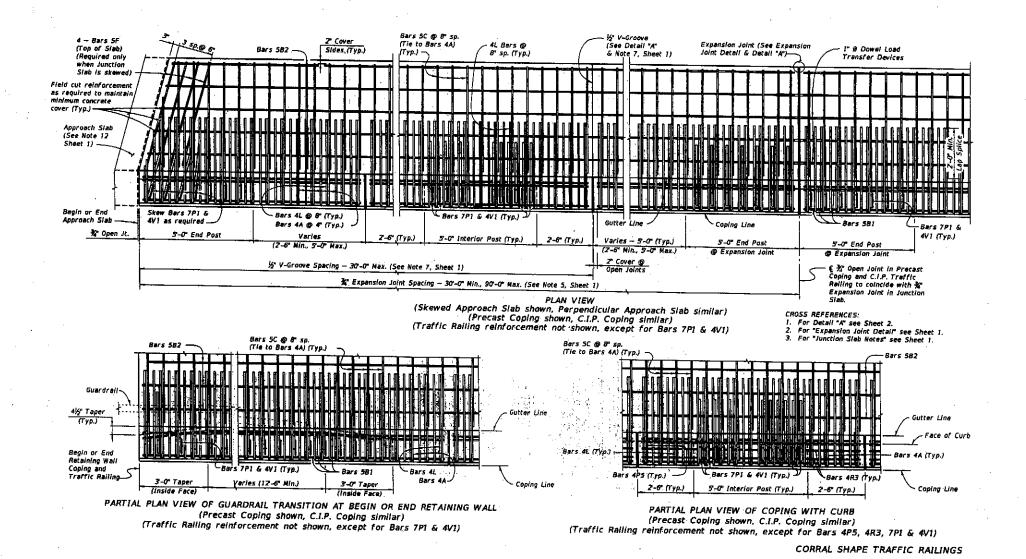
roadway superelevation.

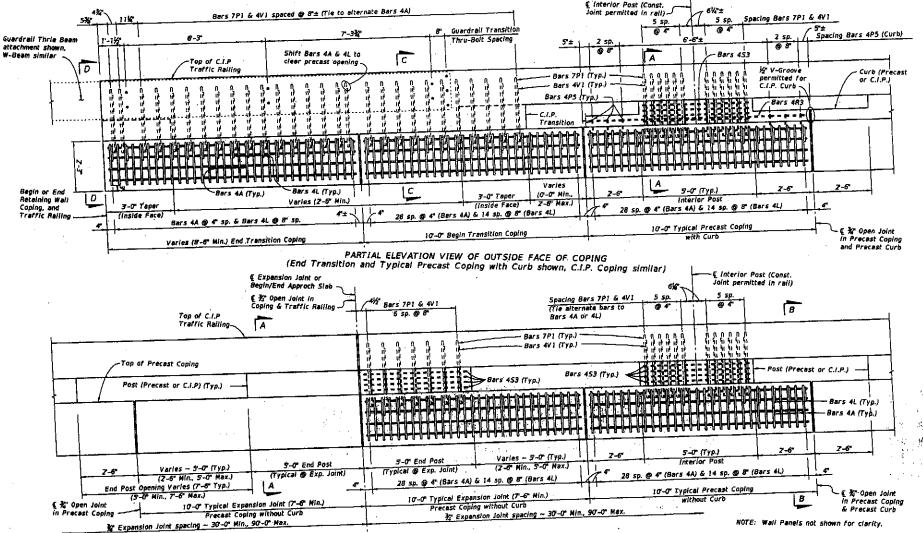
3. For Algid Pavement (Concrete), Junction Slab may be thickened to match finish grade.

Minimum length of Junction Slab between expansion joints is 30-0°.
 See Index No. 420 & 425 for additional Traffic Railing Details.

See Index No. 440 & 425 for additional Trains Rating Decisions.
 Contractor to maintain stability, of Presast Control artior to function simp completion. In the Shop Drawings, show reinforcement for optional extension required for stability, shipping and handling. Maintain 2" minimum concrete over.

7. Angle varies - 0 min., 20 max F SHAPE TRAFFIC RAILINGS





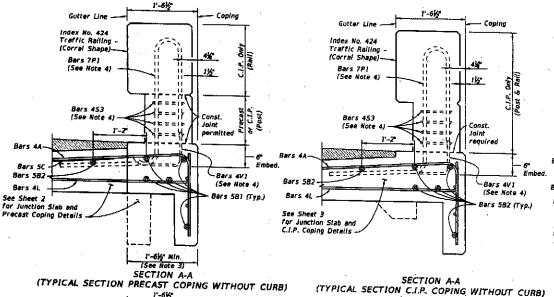
PARTIAL ELEVATION VIEW OF OUTSIDE FACE OF COPING (Precast Coping at Expansion Joint and Typical Precast Coping without Curb shown, C.I.P. Coping similar)

CROSS REFERENCES:

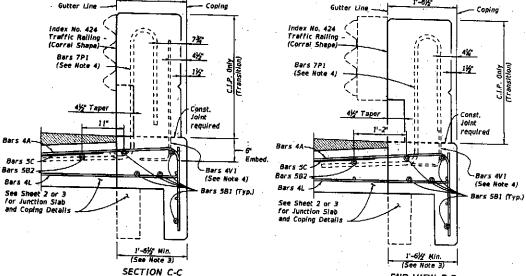
 For Sections A-A, 8-B, C-C & D-D, see Sheet 6.

2. For Junction Siab Notes, see Sheet 1.

CORRAL SHAPE TRAFFIC RAILINGS

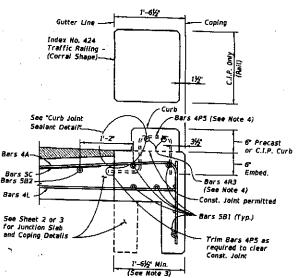


1'-615"

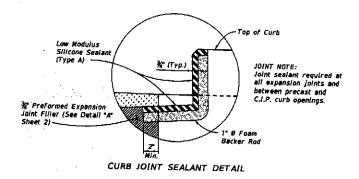


(TYPICAL SECTION TRANSITION COPING) (Precast Coping shown, C.I.P. Coping similar)

END VIEW D-D (TYPICAL SECTION TRANSITION COPING) (Precast Coping shown, C.I.P. Coping similar)



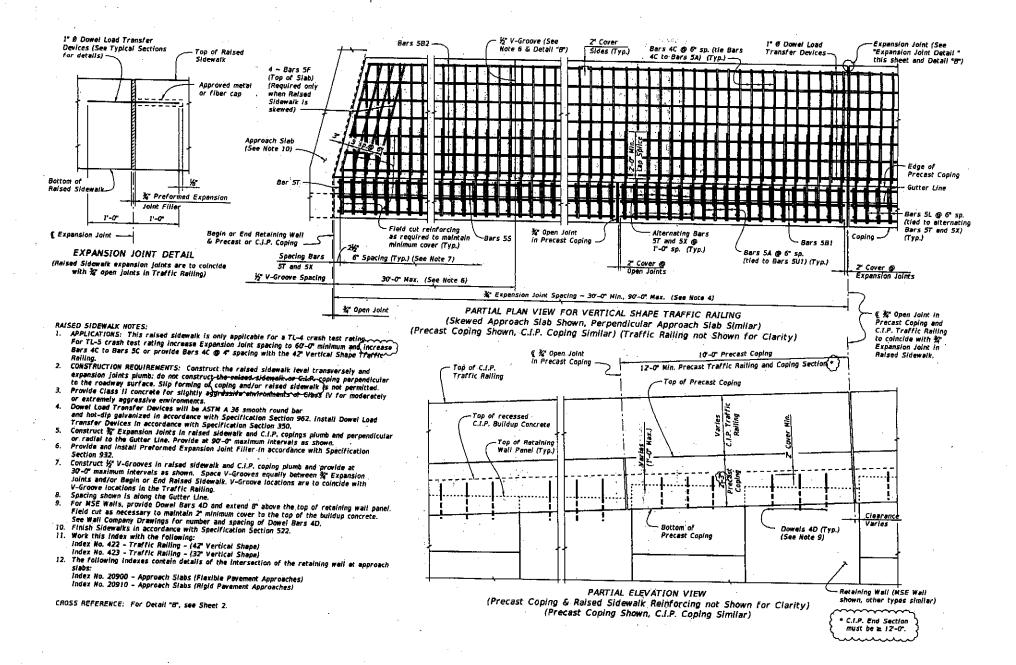
SECTION B-B (TYPICAL SECTION WITH CURB) (Precast Coping Shown, C.I.P. Coping Similar)

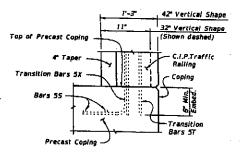


NOTES:

- See Sheets 2 & 3 for Junction Siab and additional Coping details.
 Silp Forming of C.I.P. Traffic Railling is not permitted.
- Supporting or Conference mailting is not permittee.
 Actual width varies depending on type of Retaining Wall used.
 See Index No. 424 for Traffic Railing details and Bars 7P1, 4P5, 4R3, 4S3 & 4V1. Bars 5R2 and 5U are not required in Retaining. Wall Coping.

CORRAL SHAPE TRAFFIC RAILINGS



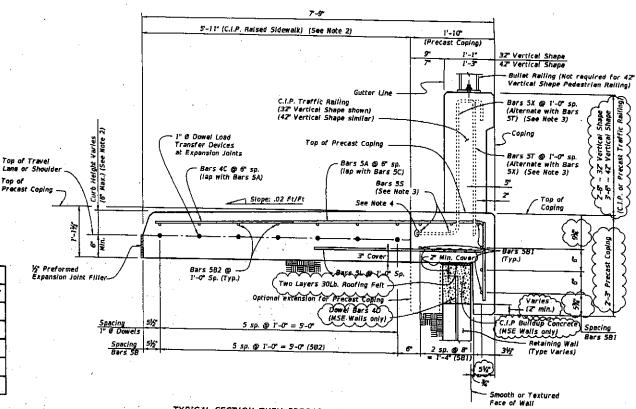


PARTIAL END VIEW OF TRAFFIC RAILING END TRANSITION FOR GUARDRAIL ATTACHMENT (Showing Bars 55, Bars 5T and Bars 5X) (Precast Coping Shown, C.I.P. Coping Similar)

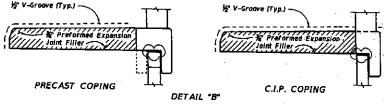
NOTE: See Index No. 422 and Index No. 423, Railing End Detail for details.

ESTIMATED QUANTITIES FOR PR	ECAST C	OPING	
ІТЕМ	UNIT	QUANTITY	
Concrete (Precast Coping)	CY/FT	0.091	
Concrete (C.I.P. Raised Sidewalk)	CY/FT.	0.232	
Reinforcing Steel (Precast Coping) excluding Bars ST, 5X and 5S (Typ.)	LB	(22.00)	
Reinforcing Steel (C.I.P. Raised Sidewalk) (Typ.)	LB/FT	11.92	
Additional Reinf. @ Expansion Joints	re.	32.04	

The above concrete quantities are based on a Type D Concrete Curb (See Note 2).



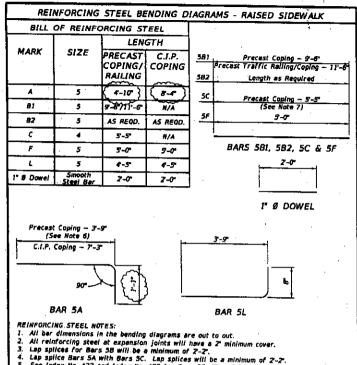
TYPICAL SECTION THRU PRECAST COPING WITH C.I.P. RAISED SIDEWALK AND RETAINING WALL AT EXPANSION JOINTS



(Showing Locations of ½" V-Grooves and ¾" Preformed Expansion Joint Filler)

NOTES:

- I. Actual width varies depending on type of Retaining Wall used.
- Match roadway curb shape (Type) and height. See Roadway Plans and Index No. 300. 5-11" dimension is based on a 32" Vertical Shape Traffic Railing with a Type D curb adjacent to a 6-0" wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.
- 3. See Index No. 422 and Index No. 423 for Bars 55, 5T & SX and Bullet Railing details. Adjust vertical dimension of Bars 5T and 5X, see Reinforcing Steel Note 5.
- 4. Trim end of Bars 57 and 5% to clear construction joint for 42 Vertical Shape Traffic Railing.
- 5. At the Contractor's option, mechanical couplers may be used to splice reinforcing, Complete details, including reinforcement lengths are required in the Shop Drawings. Mechanical couplers shall develop 125% of the bar yield strength.
- 6. Contractor to maintain stability of precast coping prior to junction stab completion.

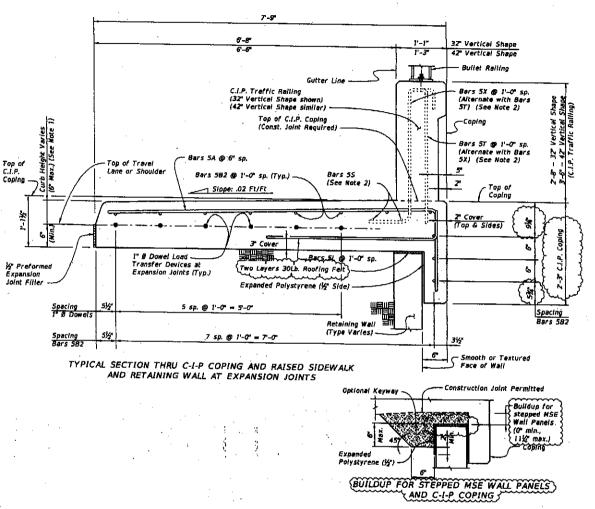


Lep splice Bars 5A With Bars 5C. Lap splices will be a minimum of 2-2.
 See Index No. 422 and Index No. 423 for Bars 5S, 5T and 5X. Adjust vertical dimensions of Stirrup Bars 5T and 5X to 3-0" for 32" Vertical Shape or 3-10" for 42" Vertical Shape.

- Dimension shown is for tap splice option. For mechanical coupler option, this dimension is 1'-8'.
- Dimension shown is far tap splice option. For mechanical coupler option, this dimension is 5-6".
- The Contractor may use Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement will conform to ASTM A 497.

ESTIMATED QUANTITIES FOR C.I.P. COPING				
ITEM UNIT QUANTIT				
Concrete	CY/FT	0.322		
Reinforcing Steel (Typical) excluding Bars 5T, 5X and 5S (Typ.)	LB/FT	(28.85)		
Additional Reinf. @ Expansion Joints	LB	32.04		

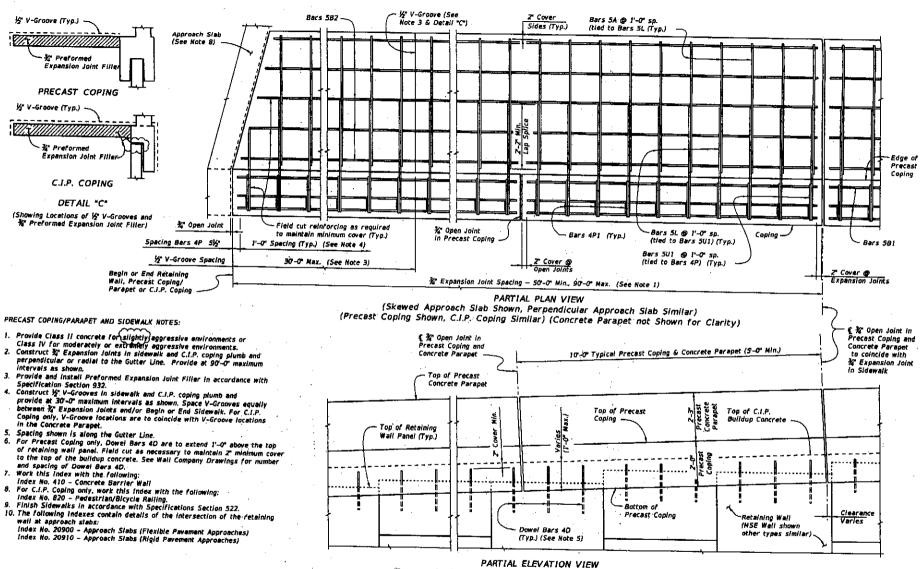
The above concrete quantities are based on a Type D Concrete (Curb on an MSE Wall) See Note 1).



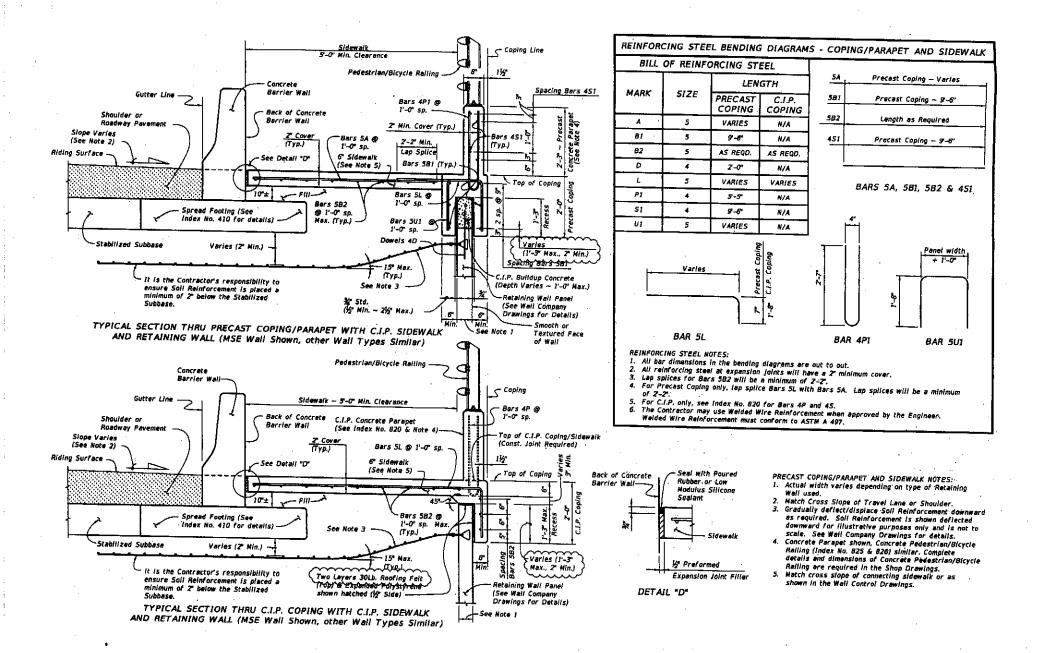
NOTES:

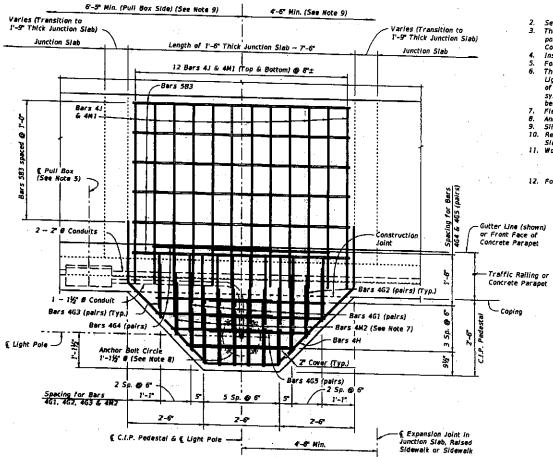
 Match roadway curb shape (Type) and height. See Roadway Plans and Index No. 300. 6-8 dimension is based on a 32 Vertical Shape Traffic Railing with a Type D curb adjacent to a 6-0° wide sidewalk. Adjust this dimension as required for other curb types or transitions at Begin or End Retaining Wall.

 See Index No. 422 and Index No. 423 for Bars 55, 5T & 5X and Bullet Railing details. Adjust vertical dimension of Bars 5T and 5X, see Reinforcing Steel Note 5.



PARTIAL ELEVATION VIEW
(Precast Coping and Sidewalk Reinforcing not Shown for Clarity)
(Precast Coping Shown, C.I.P. Coping Similar)





PLAN VIEW (Junction Slab reinforcing not shown for clarity) (Junction Slab Shown, Raised Sidewalk or Sidewalk Similar)

LIGHT PEDESTAL NOTES:

1. The pedestal and junction slab are designed to resist the following working loads from the lightpole applied at the top of the Pedestal:

Wind load Moment about Transverse Axis (*) Wind load Moment about Longitudinal Axis (*)	-	1.560 kip 40.60 kip-ft
Dead load Moment should be the the	-	28.30 k/p-ft
Dead load Moment about Longitudinal Axis (*) Maximum Shear	-	1.690 kip-ft
Torsion about Pole Axis	-	1.380 kip
(*) - Avis colors to Belden 4.1	-	3.560 kip-ft

(*) - Axis refers to Bridge Axis.
2. See Index No. 21200 for anchor bolt design and notes.

2. See Index.No. 21200 for anchor book design and notes.
3. The Contractor is responsible for ensuring the anchor bolt design is compatible with the light pole base plate. Modifications to the anchor bolt design must be signed and sealed by the Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.

For conduit, pull box and expansion/deflection fitting details, see Utility Conduit Detail Drawings. The cost of anchor boits, nuts, washers and anchor plates will be included in the Bid Price for Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, pull boxes and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Traffic Railing or Concrete Parapet that the pedestal is

Field Cut Bars 4M2 as required to maintain clearance.

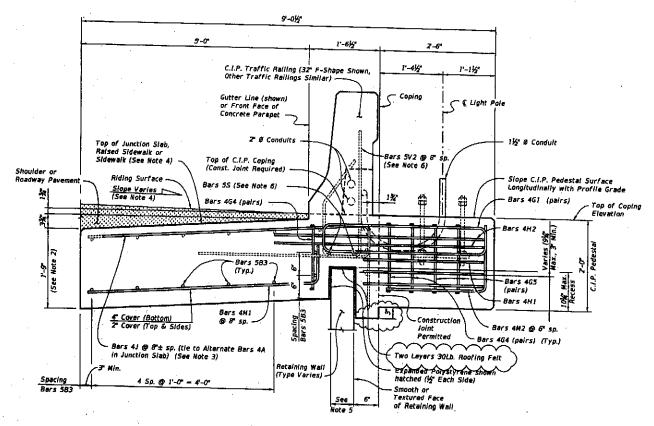
Anchor Bolt pattern orientation will be as shown.

Slip forming Method of construction requires the Engineer's approval within the limits shown. 10. Reinforcing shown for light pole pedestals is in addition to typical rainforcing for C.I.P. Junction Slabs and Raised Sidewalks.

11. Work this index with the following as appropriate:

Index No. 6110 Index No. 6120 Index No. 6130

12. For Estimated Quantities, see Sheet No. 3.



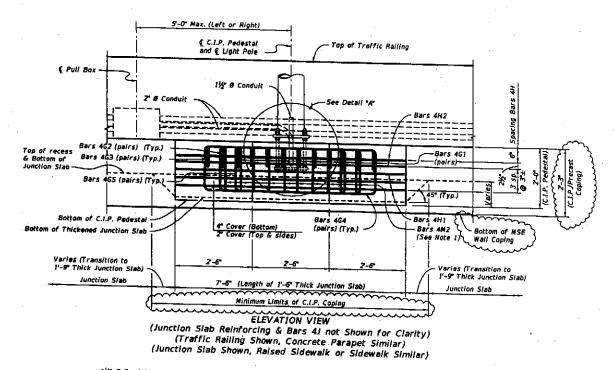
TYPICAL SECTION AT LIGHT POLE PEDESTAL (Traffic Railing Shown, Concrete Parapet Similar) (Junction Slab Shown, Raised Sidewalk or Sidewalk Similar)

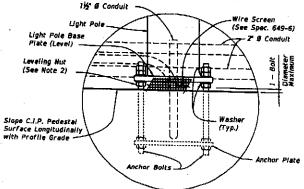
NOTES:

- 1. Provide Concrete Class to metch adjacent coping.
 2. For junction slabs, increase the 1'-0' depth dimension to 1'-9". For sidewalks For junction slabs, increase the 1'-0' depth dimension to 1'-9'. For sidewalks see Index No. 6130 for C.I.P. Coping, but increase 6' depth dimension to 1'-6'. The minimum length of the Junction Slabs, Raised sidewalks and Sidewalks is 30'-0', measured along the Gutter Line.

 Bars 41 are only required when pedestals are behind a Traffic Railing. Match the slape of the adjoining junction slab and shoulder or roadway pavement.

- (else) Suggests of Students. Actual width varies depending on type of Retaining Wall used. See Index No. 6110 for Bars 5V2 and 5S.





DETAIL "A"

NOTES:

1. Field Cut Bars 4M2 as required to maintain minimum cover. 2. Maximum clearance between leveling nut and top of pedestal will not exceed anchor boit diameter.

ESTIMATED QUAN	ITITIES	
ITEM	UNIT	QUANTITY
Concrete (Pedestal)	CY	0.926
Concrete (Thickened Junction Stab)	CY	1.222
Reinforcing Steel	Lb.	349

(The quantities above are for one C.I.P. Light Pole Pedestal. The concrete quantity for the thickened junction siab is based on a 6" increase in thickness and a 5" wide retaining well panel. Adjust thickened concrete quantity as required for raised sidewalks and sidewalks.)

REINFORCING STEEL BENDING DIAGRAMS - LIGHT POLE PEDESTAL BILL OF REINFORCING STEEL 583 7-2 MARK SIZE NO. REQD. LENGTH 4/ 6-0" 83 アーン G. 4 16 5'-8" G2 4 4 BARS 583 & 41 4'-8" G3 4 4 4'-2" G4 4 6 8-10 G5 4 4 7-4 HI 4 3 9-8 H2 4 2 13'-8" J 4 12 6-0 Z-7° 2-4" 2-7" MI 4 12 5-10 BAR 4H2 M2 10 3-8" 2-4" BAR 4H1 2-6 4G1 2-0" 4G2 5-0 1'-9" 4G.3 4M2 2-2 3-8 4G4 2-11" 4G5

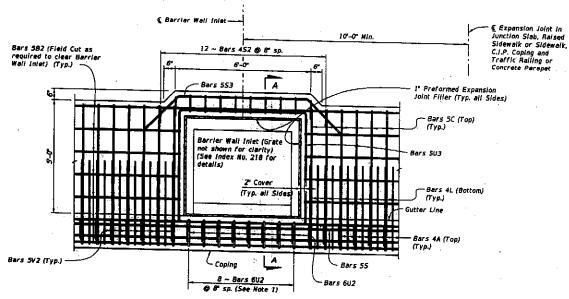
4G4 & 4G5 REINFORCING STEEL NOTES:

BARS 4G1, 4G2, 4G3,

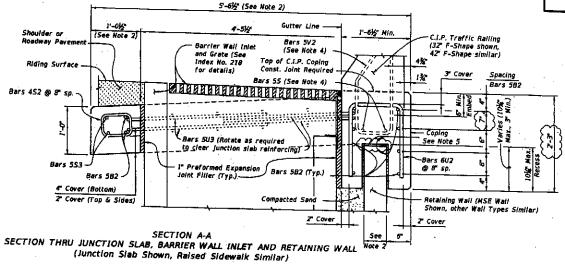
- All bar dimensions in the bending diagrams are out to out.

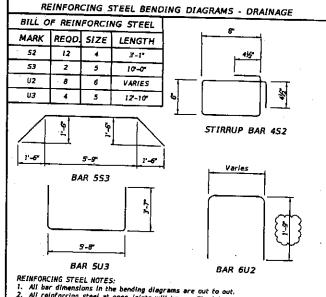
 Lap spikes for Bars 4G1, 4G2 & 4G3 will be a minimum of 1'-4". Lap splices for Bars 4G4 & 4G5 will be a minimum of 1'-8".
- The Contractor may use Welded Wire Reinforcement when approved by the Engineer, Weided Wire Reinfarcement will conform to ASTM A 497.

BAR 4MI & 4M2



PLAN VIEW (Junction Slab Shown, Raised Sidewalk Similar)





- All reinforcing steel at open joints will have a 2º minimum cover.
- See Index Nos. 6110, 6120 & 6130 for Bers 4A (or 5A), 5B, 5C end 4L (or 5L).
- The Contractor may use Welded Wire Reinforcement when approved by the Engineer. Welded Wire Reinforcement will conform to ASTN A 497.

NOTES:

- 1. Spacing shown is along the Gutter Line. Spacing shown is for C.I.P. Junction Slab. For C.I.P. Raised Sidewalks or Sidewalks, match bar spacing and size shown in Typical Sections (i.e., 11 Bars 5U2 and 15 ~ Bars 452 © 5 spacing for Raised Sidewalks).
 2. Dimensions shown are for junction slab. Increase width as required for C-I-P Raised Sidewalk and Sidewalks.

- 3. Actual location & width vary depending on type of Retaining Wall used.
 4. See-Index-Wa-61/04.6ov.Bars-5V2 and 55.
 5. Two Layers 30Lb. Roofing Felt (Top) & Expanded Polystyrene shown hatched its Cache-Sides.
- The Coord week.

 Cocate & Barrier Wall Inlet a minimum of 10-0" away from
 Expansion Joints in Junctions Stab, Raised Sidewalk or Sidewalk,
 C.I.P. Coping and Traffic Railing or Concrete Parapet.
- Work this index with the following as appropriate:
- Index No. 5110 Index No. 6120
- Index No. 6130

Williams, Wynette

From:

Sadler, David A

Sent:

Tuesday, August 02, 2011 2:34 PM

To:

Martin, Douglas T

Cc:

Hicks, Heather, Williams, Wynette

Subject:

Signature Authority

This is to delegate signature authority for documents (excluding personnel actions) to you for when I am outof the office or on business travel status form this date through July 1, 2012.

Please ensure that my office receives a copy of all correspondence signed by you for these dates.

David A. Sadler

Director, Office of Construction

Office: 850-414-5203